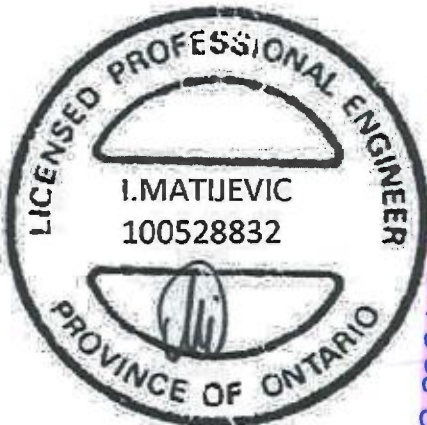


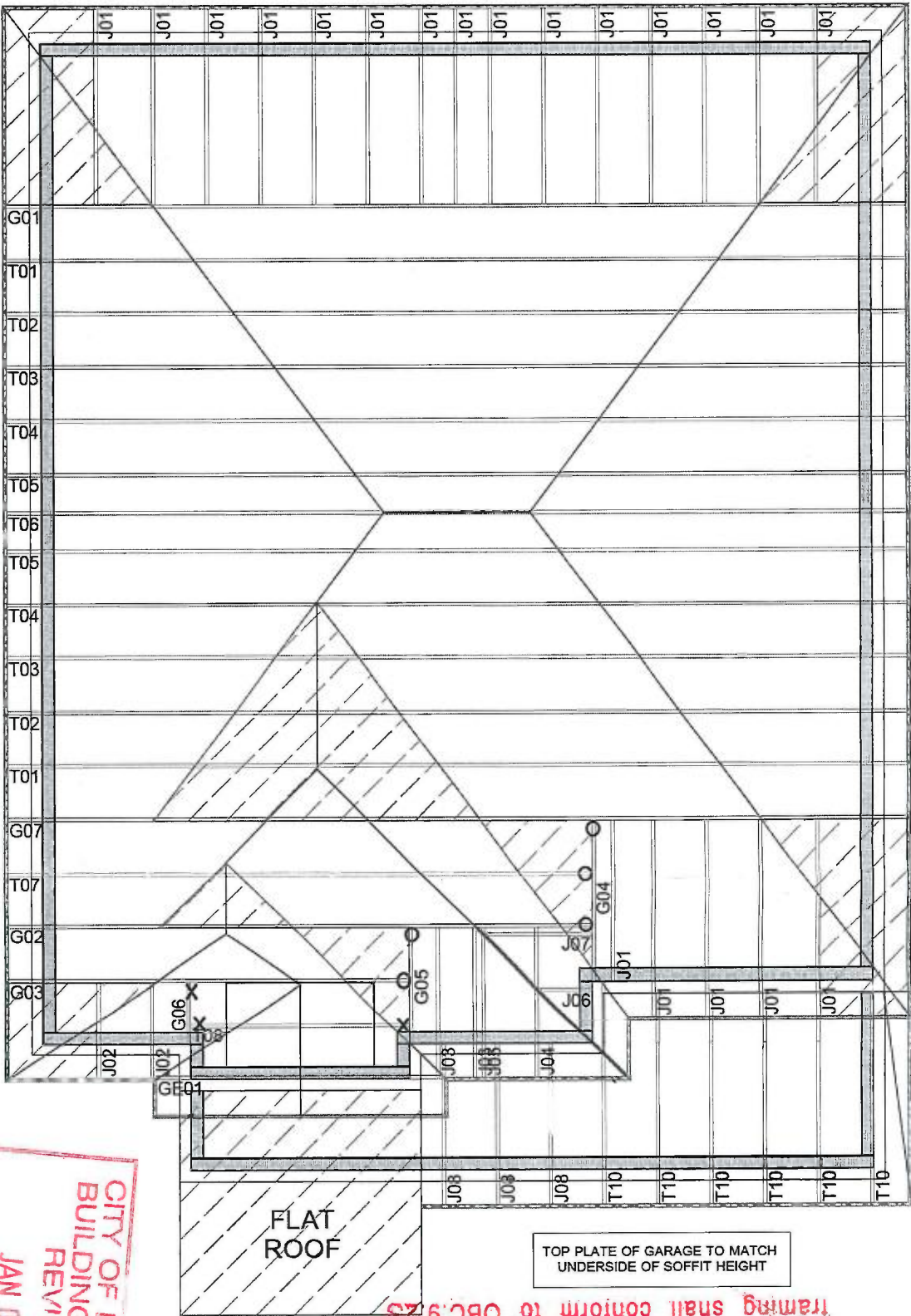
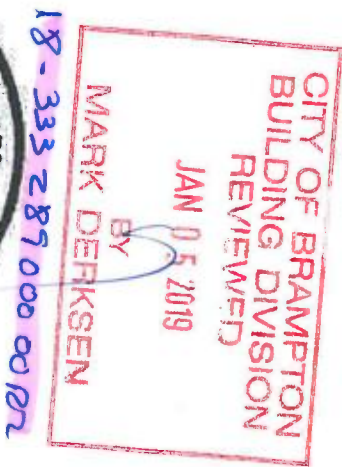


Layout based on plans provided by
JARDIN DESIGN GROUP INC
Dated April 26/18

This certification is for the engineering review of structural components shown on this drawing and is limited to the components supplied by KOTT for the loads and conditions shown on the calculation page of each component. The placement of these components on this layout is shown for reference only. Roof trusses must be installed according to manufacturer's guidelines at 24" C/C unless noted otherwise. Conventional framing shown on the layout must be designed and approved by the project engineer. Temporary and permanent bracing is to conform to BCSI guidelines.



September 14, 2018



For conventional wood framing
trussing shall conform to OBC 9.23

All work shall conform to the Ontario
Building Code O. Reg. 332/12 as amended

ROOF TRUSS LAYOUT

Lot 4

CONVENTIONAL
FRAMING BY OTHERS

HANGER LEGEND

X - LUS24 O - LJS26DS
- HGUS26 % - HGUS26-2

DESIGN CRITERIA
SPECIFIED LOADS:
TOP CH. LL = 23.3 psf
DL = 3.0 psf
BOT CH. LL = 0.0 psf
DL = 7.0 psf
TOTAL LOAD = 33.3 psf

SPACING @ 24" CENTERS,
THIS TRUSS SYSTEM IS DESIGN FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH PART 9 OF OBC 2012,
BCBC 2012, ABC 2012, CSA 086-09, TPIC 2011
ALLOWABLE DEFLECTION OF EACH COMPONENT = L/360

Model:	MILLWOOD 1 EL 1
Customer:	GREENPARK
Project:	MINNISALE HOMES
Location:	BRAMPTON
Drawn By:	DM
Date:	6/22/2018

SIZE AND LOCATION OF CONVENTIONAL FRAMING IS APPROXIMATE. ALL AREAS
MAY NOT BE SHOWN. REFER TO ARCHITECTURAL PLANS FOR DETAILS.

ALL CONVENTIONAL FRAMING TO CONFORM WITH PART 9 OF THE O.B.C. ROOF RAFTER THAT CROSS MEET
OVER TRUSSES TO BE 2X4 SPF @24" O/C WITH VERTICAL POST TO THE TRUSS BELOW EACH CROSS POINT.
VERTICAL POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END POINT
AND ROWS OF BRACING DOES NOT EXCEED 6'.

ENGINEERING NOTE PAGE (ENP-1)
PLEASE READ PRIOR TO INSTALLATION

RESPONSIBILITIES

THIS DESIGN IS FOR AN INDIVIDUAL BUILDING COMPONENT AND HAS BEEN BASED ON INFORMATION PROVIDED BY THE DESIGN OFFICE OF KOTT LUMBER. THE UNDERSIGNED ENGINEER DISCLAIMS ANY RESPONSIBILITY FOR DAMAGES AS A RESULT OF FAULTY OR INCORRECT INFORMATION, SPECIFICATION AND/OR DESIGNS FURNISHED TO THE ENGINEER. THE UNDERSIGNED ENGINEER IS ONLY RESPONSIBLE FOR THE STRUCTURAL INTEGRITY OF THIS BUILDING COMPONENT FOR THE CONDITIONS AND LOADS SHOWN ON THIS DRAWING. THE STRUCTURAL INTEGRITY OF THE BUILDING AND THE VERIFICATION OF THE DIMENSIONS AND THE DESIGN LOADS USED ARE THE RESPONSIBILITY OF THE BUILDING DESIGNER.

TRUSSES ARE DESIGNED IN CONFORMANCE WITH THE RELEVANT SECTIONS OF THE NATIONAL BUILDING CODE OF CANADA OR THE CANADIAN CODE FOR FARM BUILDINGS, WHICHEVER APPLIES TO THE BUILDING TYPE INDICATED ON THE DRAWING

IT IS THE RESPONSIBILITY OF KOTT LUMBER TO ENSURE THAT TRUSSES ARE MANUFACTURED IN CONFORMANCE WITH THESE DESIGNS AND WITH THE SPECIFICATIONS OUTLINED BELOW. THE UNDERSIGNED ENGINEER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

USE AND OCCUPANCY

- The building is of the type indicated on the drawing

LOADING

- The truss loading intensity and distribution as well as load transfer mechanism is that indicated on the drawing
- No buildings, trees, parapets or other projections higher than the roof for which the trusses are used are located within a distance less than ten (10) times the difference in height, or five metres (16 ft) whichever is greater, unless the drawing indicates that the snow drifting has been taken into account

HANDLING, INSTALLATION AND BRACING

- The trusses must be handled and installed by a qualified professional as per the supplied document titled *Information for Truss Installers* and the BCSI-B1 and BCSI-B3 Summary Sheets
- The compression chords are laterally braced by continuous rigid diaphragm sheathing or as specified on the drawing
- Temporary and permanent bracing must be installed as indicated on the truss drawing and according to the BCSI-B1 and BCSI-B3 Summary Sheets. Bracing for the lateral stability of the truss is to be provided by the building designer
- It is recommended that a Professional Engineer's advice be obtained for the bracing of trusses spanning more than 12.37m (40'-7")

SUPPORTS

- The trusses are to be supported at the bearing points indicated and anchored to the supports where considered necessary by the designer of the overall structure
- Bearing sizes shown are the minimum required to prevent crushing of the truss members and do not necessarily take into account stability of the overall building structure
- Elevation of bearings must be carefully checked and shimmed to alignment for solid bearings
- Adequate wood truss bearing is the responsibility of the building designer.

DIMENSIONS

- Geometry of the truss and dimensions indicated on the drawing are identical to those of the installed truss.



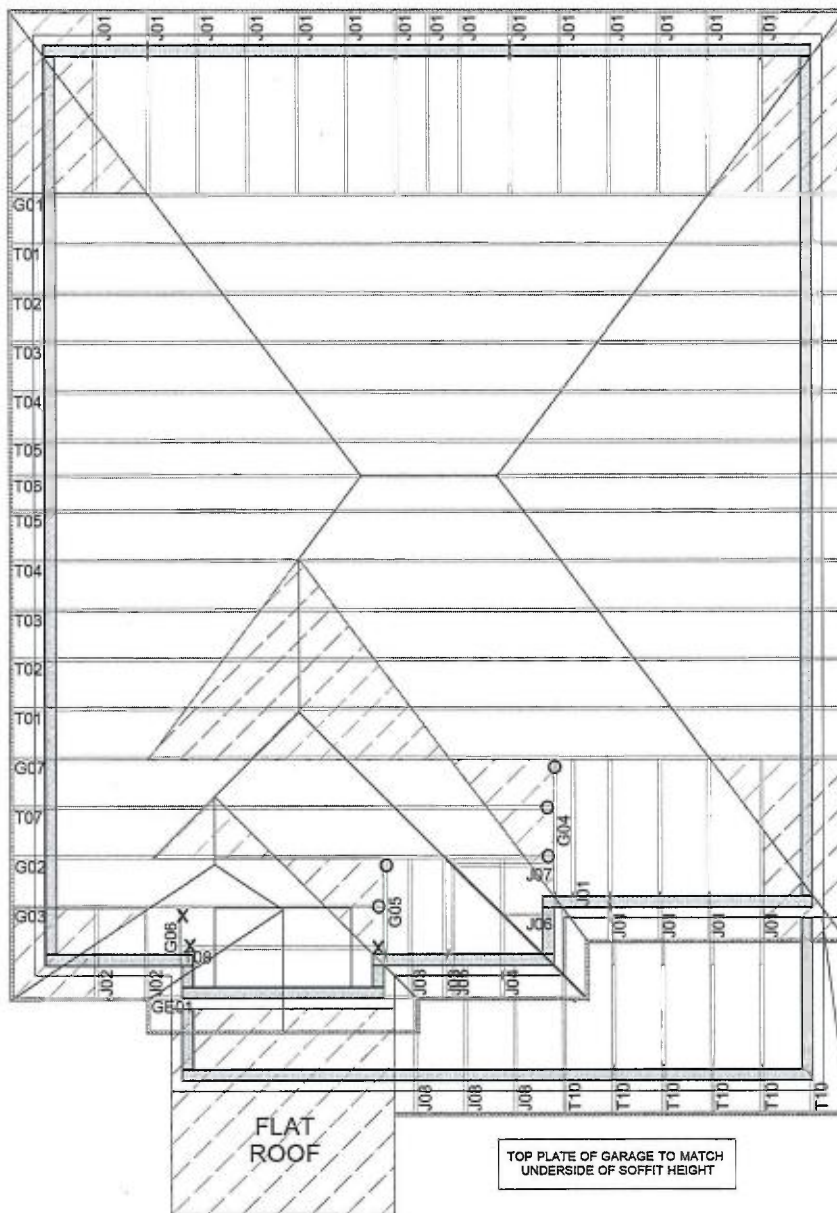


Layout based on plans provided by
JARDIN DESIGN GROUP INC
Dated April 26/18

This certification is for the engineering review of structural components shown on this drawing and is limited to the components supplied by KOTT for the loads and conditions shown on the calculation page of each component. The placement of these components on this layout is shown for reference only. Roof trusses must be installed according to manufacturer's guidelines at 24" C/C unless noted otherwise. Conventional framing shown on the layout must be designed and approved by the project engineer. Temporary and permanent bracing is to conform to BCSI guidelines.



September 14, 2018



ROOF TRUSS LAYOUT



**CONVENTIONAL
FRAMING** BY OTHERS

HANGER LEGEND

X - LUS24
- HGUS26

O - LJS26DS
% - HGUS26-2

DESIGN CRITERIA
SPECIFIED LOADS:
TOP CH. LL = 23.3 psf
DL = 3.0 psf
BOT CH. LL = 0.0 psf
DL = 7.0 psf
TOTAL LOAD = 33.3 psf

SPACING @ 24" CENTERS.
THIS TRUSS SYSTEM IS DESIGN FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH PART 9 OF OBC 2012,
BCBC 2012, ABC 2012, CSA 086-09, TPIC 2011

ALLOWABLE DEFLECTION OF EACH COMPONENT = L/360

**SIZE AND LOCATION OF CONVENTIONAL FRAMING IS APPROXIMATE. ALL AREAS
MAY NOT BE SHOWN. REFER TO ARCHITECTURAL PLANS FOR DETAILS.**

ALL CONVENTIONAL FRAMING TO CONFORM WITH PART 9 OF THE O.B.C. ROOF RAFTER THAT CROSS MEET
OVER TRUSSES TO BE 2X4 SPF @24" O/C WITH VERTICAL POST TO THE TRUSS BELOW EACH CROSS POINT.
VERTICAL POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END POINT
AND ROWS OF BRACING DOES NOT EXCEED 6'.

Model: MILLWOOD 1 EL 1

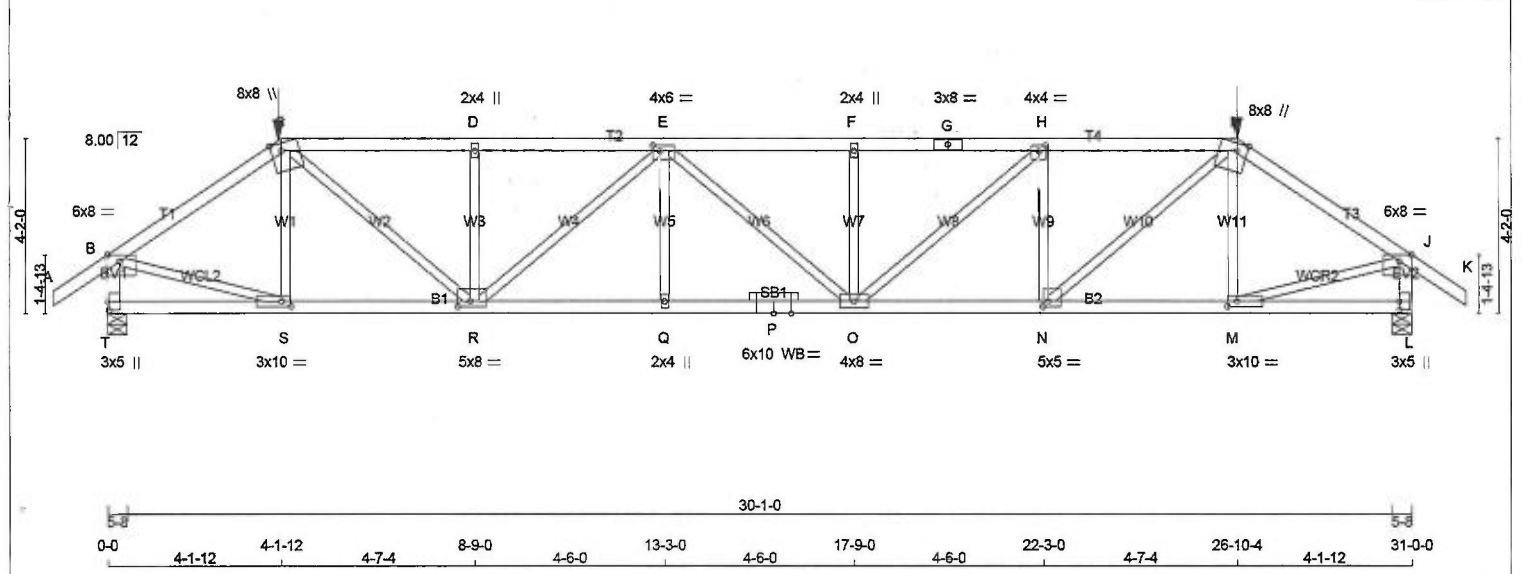
Customer: GREENPARK

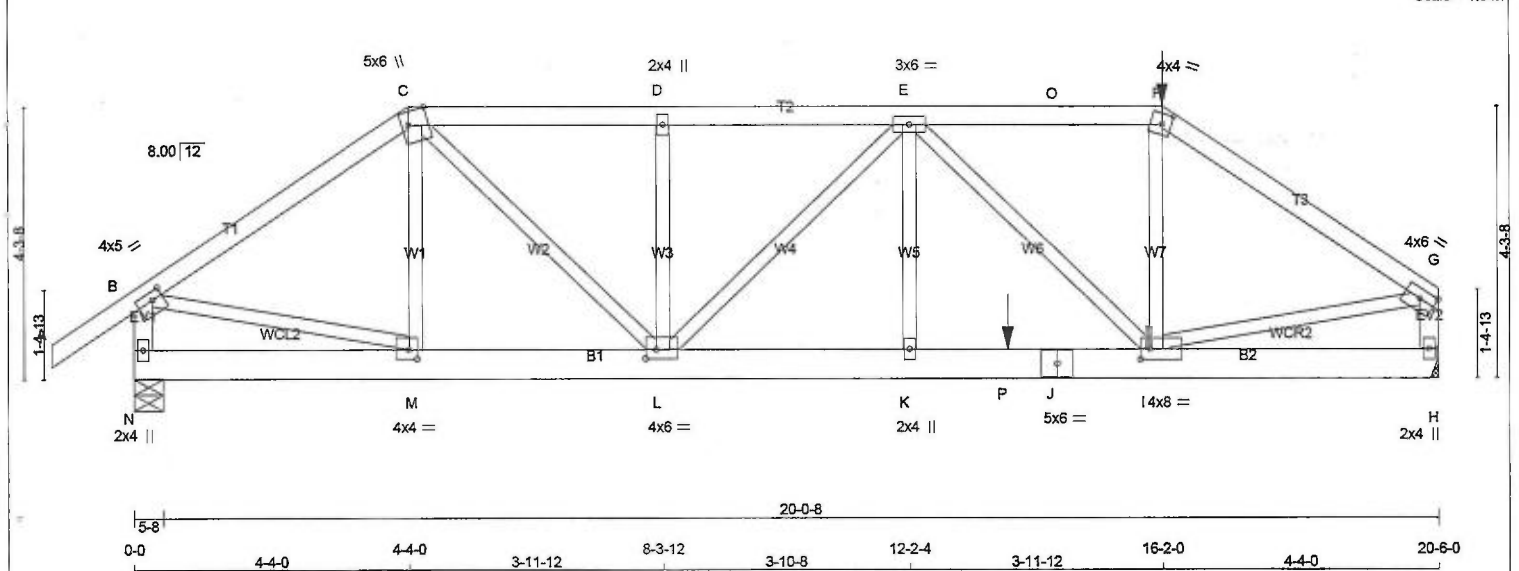
Project: MINNISALE HOMES

Location: BRAMPTON

Drawn By: DM

Date: 6/22/2018





LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
N - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	5.0	1.50	2.00
C	TTWW-t	MT20	5.0	6.0	Edge	3.75
D	TMVW-w	MT20	2.0	4.0		
E	TMVWW-t	MT20	3.0	6.0		
F	TTW-w	MT20	4.0	4.0	2.00	1.75
G	TMVW-t	MT20	4.0	6.0	1.75	Edge
H	BMV1+p	MT20	2.0	4.0		
I	BMVWW-t	MT20	4.0	8.0	2.00	1.75
J	BS-t	MT20	5.0	6.0		
K	BMVW-w	MT20	2.0	4.0		
L	BMVWW-t	MT20	4.0	6.0	1.75	2.00
M	BMVW-t	MT20	4.0	4.0	1.75	1.50
N	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES
EDGE OF CHORD

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 198.3 lbs FACTORED DOWN AT 16-2-0 ON TOP CHORD, AND 701.0 lbs FACTORED DOWN AT 13-8-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
N	1366	0	1366	0
H	1654	0	1654	0

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT H TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

1ST CASE	MAX/MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM LIVE WIND DEAD SOIL
N	956	684 / 0 0 / 0 0 / 0 272 / 0 0 / 0
H	1161	810 / 0 0 / 0 0 / 0 351 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.39 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 29	-77.4	-77.4 0.11 (1)	10.00	M-C	-161 / 19	0.04 (1)
B-C	-1426 / 0	-77.4	-77.4 0.33 (1)	5.06	C-L	0 / 933	0.23 (1)
C-D	-1846 / 0	-77.4	-77.4 0.21 (1)	4.70	L-D	-320 / 0	0.09 (1)
D-E	-1847 / 0	-77.4	-77.4 0.23 (1)	4.68	L-E	-499 / 0	0.25 (1)
E-O	-1670 / 0	-77.4	-77.4 0.25 (1)	4.85	K-E	0 / 554	0.14 (1)
O-F	-1670 / 0	-119.5	-119.5 0.25 (1)	4.85	E-I	-757 / 0	0.38 (1)
F-G	-1878 / 0	-77.4	-77.4 0.37 (1)	4.39	I-F	0 / 597	0.15 (1)
N-B	-1334 / 0	0.0	0.0 0.15 (1)	6.97	B-M	0 / 1214	0.30 (1)
H-G	-1632 / 0	0.0	0.0 0.18 (1)	6.44	I-G	0 / 1683	0.42 (1)
N-M	0 / 0	-17.5	-17.5 0.04 (4)	10.00			
M-L	0 / 1180	-17.5	-17.5 0.20 (1)	10.00			
L-K	0 / 2197	-17.5	-17.5 0.42 (1)	10.00			
K-P	0 / 2197	-17.5	-17.5 0.66 (1)	10.00			
P-J	0 / 2197	-17.5	-17.5 0.66 (1)	10.00			
J-I	0 / 2197	-25.7	-25.7 0.66 (1)	10.00			
I-H	0 / 0	-25.7	-25.7 0.12 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
F	16-2-0	-158	-158	---	FRONT	VERT	TOTAL
P	13-8-12	-701	-701	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***

GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.

LOADS WERE DERIVED FROM USER INPUT

NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF
DL = 3.0 PSF

BOT CH. LL = 0.0 PSF
DL = 7.0 PSF

TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK: 4-4-0
END SETBACK: 4-4-0
END WALL WIDTH: 5-8
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDTL LOADS BASED ON 55% OF GSL
LOADS APPLIED TO FIRST 8-0 OF SPAN MEASURED FROM THE RIGHT.

*** NON STANDARD GIRDER ***

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/999 (0.12")

CSI: TC=0.37/1.00 (F-G:1), BC=0.66/1.00 (I-K:1), WB=0.42/1.00 (G-I:1), SSI=0.30/1.00 (I-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

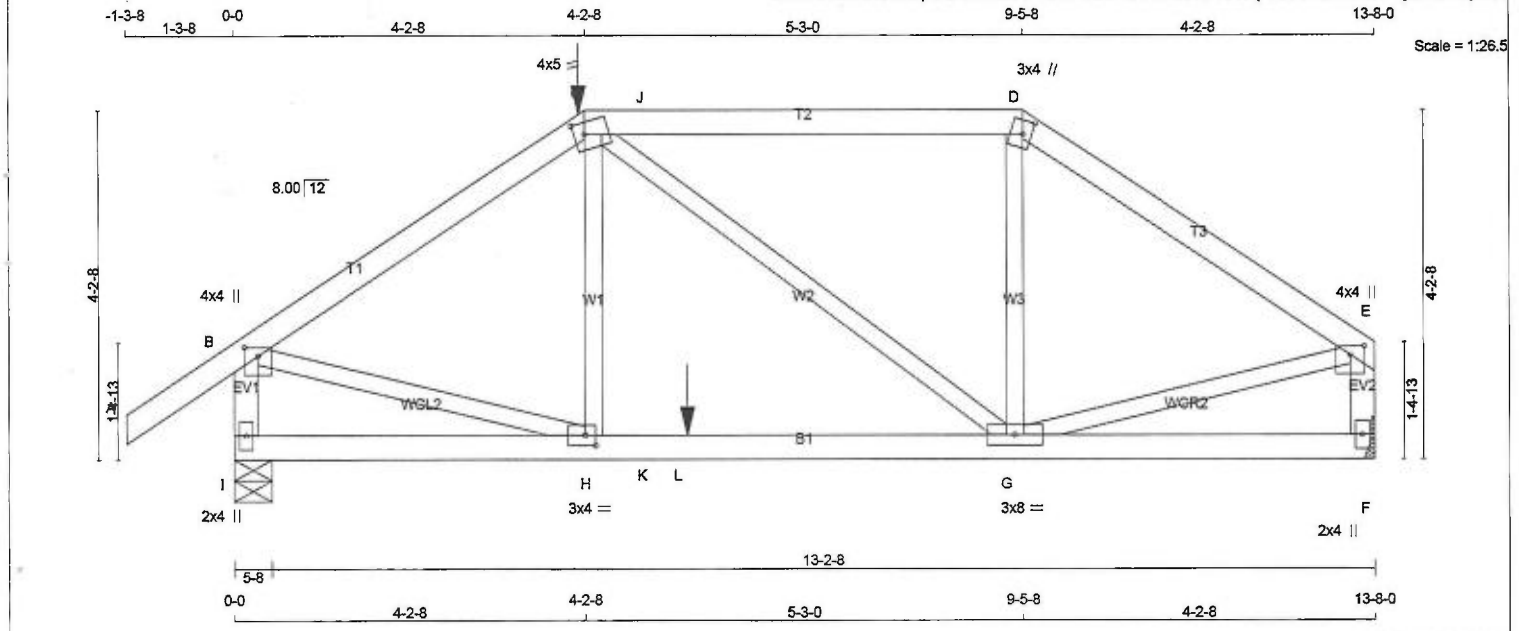
JSI GRIP= 0.90 (G) (INPUT = 0.90)
JSI METAL= 0.57 (J) (INPUT = 1.00)



September 14, 2018

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER				DESIGNER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
CHORDS	SIZE			GROSS REACTION	GROSS REACTION	BRG	BRG
A - C	2x4	DRY No.2	SPF	VERT	DOWN	UPLIFT	IN-SX
C - D	2x4	DRY No.2	SPF	HORZ	HORZ	5-8	IN-SX
D - E	2x4	DRY No.2	SPF				1-10
I - B	2x4	DRY No.2	SPF				MECHANICAL
F - E	2x4	DRY No.2	SPF				
I - F	2x4	DRY No.2	SPF				
ALL WEBS EXCEPT	2x3	DRY No.2	SPF				

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING				DESIGN CRITERIA			
UNFACTORED REACTIONS				SPECIAL LOADS ANALYSIS			
1ST LCASE	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL	GEOMETRY AND/OR BASIC LOADS CHANGED BY USER
JT	671	487 / 0	0 / 0	0 / 0	164 / 0	0 / 0	LOADS WERE DERIVED FROM USER INPUT
F	544	383 / 0	0 / 0	0 / 0	161 / 0	0 / 0	NO FURTHER MODIFICATIONS WERE MADE

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I				SPECIFIED LOADS:			
BRACING				TOP CH. LL = 23.3 PSF			
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.				DL = 3.0 PSF			
LOADING				BOT CH. LL = 0.0 PSF			
TOTAL LOAD CASES: (4)				DL = 7.0 PSF			
				TOTAL LOAD = 33.3 PSF			

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRAC LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRAC LENGTH (FT)	
FR-TO				FR-TO			
A-B	0 / 29	-77.4	0.11 (1)	10.00	H-C	0 / 151	0.05 (4)
B-C	-894 / 0	-77.4	0.28 (1)	8.11	C-G	-123 / 0	0.09 (1)
C-J	-648 / 0	-78.2	0.42 (1)	6.25	G-D	0 / 91	0.03 (4)
J-D	-648 / 0	-77.4	0.42 (1)	6.25	B-H	0 / 766	0.19 (1)
D-E	-778 / 0	-77.4	0.28 (1)	6.25	G-E	0 / 867	0.17 (1)
I-B	-953 / 0	0.0	0.0 (1)	7.81			
F-E	-758 / 0	0.0	0.0 (1)	7.81			

FACTORED CONCENTRATED LOADS (LBS)				NAIL VALUES			
JT	LOC.	LC1	MAX	PLATE	GRIP (DRY)	SHEAR	SECTION
C	4-2-8	-54	-54	---	(PSI)	(PLI)	(PLI)
L	5-5-4	-278	-278	---	MAX MIN	MAX MIN	MAX MIN

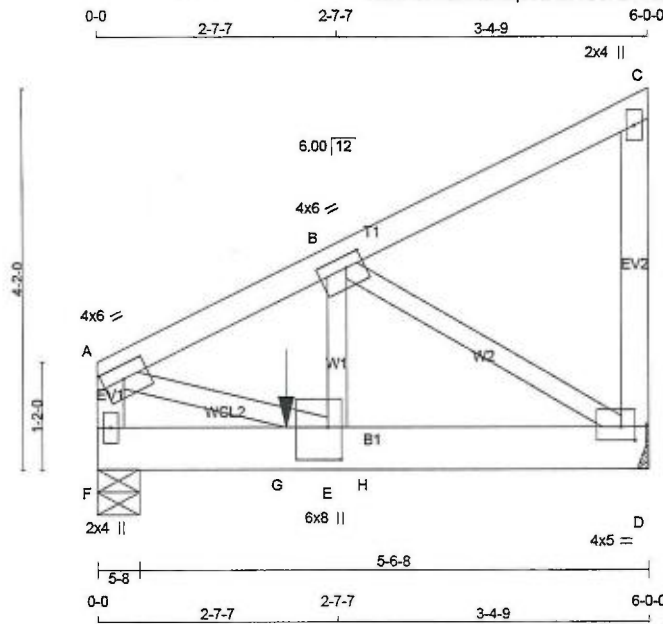
September 14, 2018

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Kott, Ottawa, Ontario, IM

Version 8.210 S Mar 12 2018 MitTek Industries, Inc. Fri Sep 14 08:06:06 2018 Page 1

ID:GvG8oeB5QNkqs?axTtV?B3Fz4ISM-Mbw1wY1?rJdNWE9XpohC41?LDfoWBTgJCVC7uydnf



Scale: 1/2"=1'

TOTAL WEIGHT = 29 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
D - C	2x4	DRY	No.2
F - A	2x4	DRY	No.2
F - D	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

SPF

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0	1.75	2.75
B	TMVW-t	MT20	4.0	6.0	1.50	2.25
C	TMV+p	MT20	2.0	4.0		
D	BMVW1-t	MT20	4.0	5.0	1.75	1.75
E	BMVW1-t	MT20	6.0	8.0	4.25	1.75
F	BMV1+p	MT20	2.0	4.0		

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1654.3 lbs FACTORED DOWN AT 2-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
D	1816	0	1816	0	0	0
F	1691	0	1691	0	5-8	2-14

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT D TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS				
		SNOW	LIVE	PERM.LIVE	WIND	DEAD
D	1275	892 / 0	0 / 0	0 / 0	0 / 0	383 / 0
F	1187	830 / 0	0 / 0	0 / 0	0 / 0	357 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.71 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		MAX. FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MAX. UNBRACED LENGTH	MEMB.	WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED					MAX. FACTORED FORCE (LBS)	FACTORED
FR-TO						FR-TO		
A-B	-1935 / 0	-77.4	-77.4	0.13 (1)	4.71	E-B	0 / 1674	0.41 (1)
B-C	-7 / 0	-77.4	-77.4	0.13 (1)	10.00	B-D	-2042 / 0	0.54 (1)
D-C	-116 / 0	0.0	0.0	0.03 (1)	7.81	A-E	0 / 1813	0.45 (1)
F-A	-1466 / 0	0.0	0.0	0.16 (1)	6.72			
F-G	0 / 0	-17.5	-17.5	0.31 (1)	10.00			
G-E	0 / 0	-17.5	-17.5	0.31 (1)	10.00			
E-H	0 / 1739	-17.5	-17.5	0.55 (1)	10.00			
H-D	0 / 1739	-445.4	-445.4	0.55 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1 MAX-	MAX+	FACE	DIR.	TYPE
G	2-0-12	-1654	-1654	—	BACK	VERT

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	PSF
LL	23.3	PSF
DL	3.0	PSF
BOT CH.	LL	PSF
LL	0.0	PSF
DL	7.0	PSF
TOTAL LOAD	33.3	PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStd Girder

START DISTANCE = 3-0-0
START SPAN CARRIED = 20-6-0
END DISTANCE = 6-0-0
END SPAN CARRIED = 20-6-0
END WALL WIDTH = 5-8
APPLIED TO BACK SIDE OF BOTTOM CHORD.
- ADDTL LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.16/1.00 (A-F:1), BC=0.55/1.00 (D-E:1), WB=0.54/1.00 (B-D:1), SS=0.98/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 616 354 1667 622 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

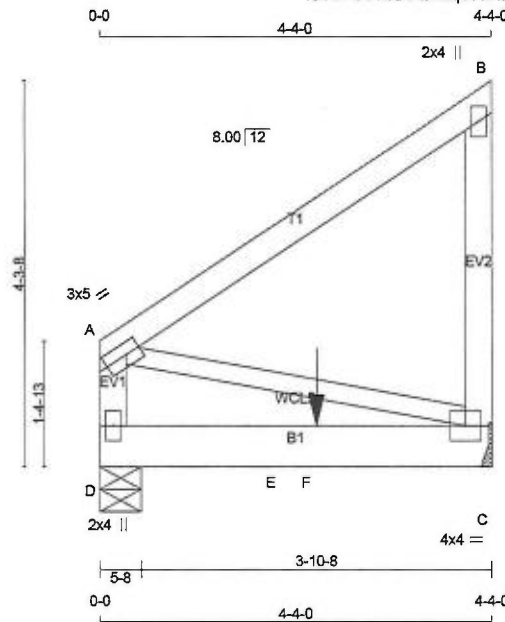
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (B) (INPUT = 0.90)
JSI METAL= 0.52 (D) (INPUT = 1.00)



September 14, 2018

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



Scale = 1:24.5

TOTAL WEIGHT = 22 lb

LUMBER
N.L.G.A. RULES
CHORDS SIZE LUMBER
A - B 2x4 DRY No.2
C - B 2x4 DRY No.2
D - A 2x4 DRY No.2
D - C 2x6 DRY No.2
ALL WEBS 2x3 DRY No.2
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)
JT TYPE PLATES W LEN Y X
A TMVW-t MT20 3.0 5.0 1.50 2.00
B TMVW-p MT20 2.0 4.0
C BMVW1-t MT20 4.0 4.0
D BMVW1-p MT20 2.0 4.0

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 775.1 lbs FACTORED DOWN AT 2-4-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	BRG	IN-SX
C	702	0	702	0	0	5-8	1-8	
D	777	0	777	0	0			

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT C TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN.	COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL
C	492	344 / 0 0 / 0 0 / 0 0 / 0 148 / 0 0 / 0
D	545	382 / 0 0 / 0 0 / 0 0 / 0 164 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. UNBRACED LENGTH	FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	FR-TO
A-B	0 / 0	-77.4	-77.4	0.27 (1)	10.00	A-C	0 / 0	0.00 (1)	
C-B	-168 / 0	0.0	0.0	0.04 (1)	7.81				
D-A	-168 / 0	0.0	0.0	0.02 (1)	7.81				
D-E	0 / 0	-163.7	-163.7	0.74 (1)	10.00				
E-F	0 / 0	-17.5	-17.5	0.74 (1)	10.00				
F-C	0 / 0	-17.5	-17.5	0.74 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
F	2-4-12	-775	-775	--	BACK	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CSldGirder

START DISTANCE = 0-0
START SPAN CARRIED = 8-2-0
END DISTANCE = 2-0-0
END SPAN CARRIED = 8-2-0
END WALL WIDTH = 0-0
APPLIED TO BACK SIDE OF BOTTOM CHORD.
- ADDTL LOADS BASED ON 55% OF GSL.

*** NON STANDARD GIRDER ***

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/600 (0.09")

CSI: TO=0.27/1.00 (A-B:1), BC=0.74/1.00 (C-D:1), WB=0.00/1.00 (A-C:1), SSI=0.40/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (A) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)



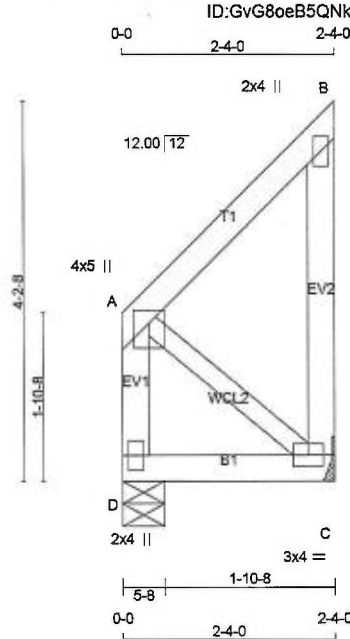
September 14, 2018

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



JOB NAME IM0918-064 Kott, Ottawa, Ontario, IM	TRUSS NAME G06	QUANTITY 1	PLY 1	JOB DESC. TR - GREENPARK - MINNISALE HOMES - MILLWOOD 1 ELE 1 TRUSS DESC.	DRWG NO. Page 8 of 29 IM0918-064
---	-------------------	---------------	----------	---	-------------------------------------

Version 8.210 S Mar 12 2018 MTEK Industries, Inc. Fri Sep 14 08:06:07 2018 Page 1



Scale = 1:24.4

TOTAL WEIGHT = 14 lb (M)

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF	
C - B	2x4	DRY	No.2	SPF	
D - A	2x4	DRY	No.2	SPF	
D - C	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	5.0	1.75	2.00	
B	TMV+p	MT20	2.0	4.0			
C	BMVW1-t	MT20	3.0	4.0			
D	BMV1+p	MT20	2.0	4.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING

DESIGNER BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
C	278	0	278	0
D	278	0	278	0

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT C TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN	COMPONENT REACTIONS	DEAD	SOIL
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND
C	195	136/0	0/0	0/0	0/0
D	195	136/0	0/0	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED MEMB. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. UNBRACED LENGTH	WEBS	MAX. FACTORED MEMB. FORCE (LBS)	FACTORED MAX. CSI (LC)
FR-TO					FR-TO		
A-B	0/0	-77.4	-77.4	0.08 (1)	10.00	A-C	0/0
C-B	-90/0	0.0	0.0	0.02 (1)	7.81		0.00 (1)
D-A	-90/0	0.0	0.0	0.01 (1)	7.81		
D-C	0/0	-160.8	-160.8	0.16 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:	
TOP CH. LL	= 23.3 PSF
DL	= 3.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 33.3 PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 8-2-0
END DISTANCE = 2-4-0
END SPAN CARRIED = 8-2-0
END WALL WIDTH = 1-8
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDTL LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.08/1.00 (A-B-1), BC=0.16/1.00 (C-D-1),
WB=0.00/1.00 (A-C-1), SS=0.16/1.00 (C-D-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

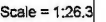
JSI GRIP= 0.09 (C) (INPUT = 0.90)
JSI METAL= 0.03 (B) (INPUT = 1.00)



September 14, 2018

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

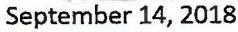




TOTAL WEIGHT = 40 lb
[M][F]

PLATES (table is in inches)					
J	TYPE	PLATES	W	LEN	Y X
B	TMVW-I	MT20	3.0	5.0	1.50 2.00
C	TMVW-I	MT20	3.0	4.0	1.50 1.50
D	TMVW-I	MT20	3.0	4.0	2.25 1.50
E	TMVW-I	MT20	3.0	4.0	1.50 1.50
F	TMVW-I	MT20	3.0	5.0	1.50 2.00
G	BMVW-I	MT20	3.0	4.0	
H	BMV+P	MT20	2.0	4.0	
J	BVMWV-I	MT20	4.0	6.0	3.00 4.00
K	BMVWV-I	MT20	3.0	6.0	
L	BMVWV-I	MT20	4.0	6.0	3.00 4.00
M	BMVWV-I	MT20	2.0	4.0	
N	BMVWV-I	MT20	3.0	4.0	
O	P,Q,R				
O	NP+W	MT20	2.0	4.0	

PLATE ROTATION TOL. = 5.0 Deg.
JSI GRIP= 0.52 (F) (INPUT = 0.90)
JSI METAL= 0.19 (F) (INPUT = 1.00)

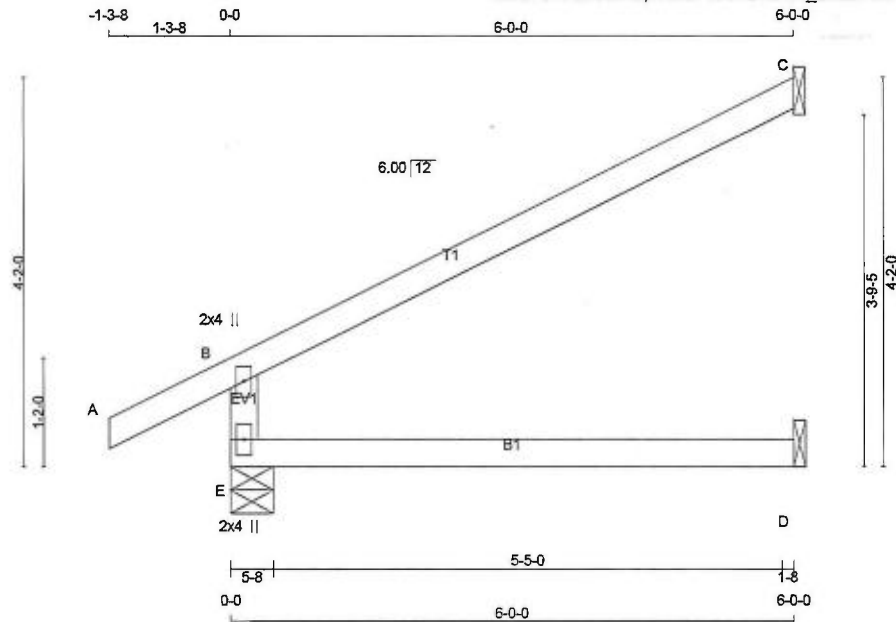


KOTT

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
IM0918-064	J01	20	1	TR - GREENPARK - MINNISALE HOMES - MILLWOOD 1 ELE 1	Page 11 of 29 IM0918-064
Kott, Ottawa, Ontario, IM					

ID:GvG8oeB5QNkqs?axtV?B3Fz4ISM-I_2oLE3GMwt4IYJwxDjg9S4cpTabfVY?fpBJCnydntD

Version 8.210 S Mar 12 2018 Mitek Industries, Inc. Fri Sep 14 08:06:08 2018 Page 1



Scale = 1:23.6

TOTAL WEIGHT = 20 X 17 = 342 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	2.0	4.0
E	BMV1+p	MT20	2.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING

DESIGNER BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
VERT	HORZ	DOWN	HORZ	UPLIFT
E	457	0	457	0
C	174	0	174	0
D	43	0	49	0

SEE MITEK STANDARD DETAIL 837579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN	COMPONENT REACTIONS	WIND	DEAD	SOIL
COMBINED	SNOW	LIVE	PERM	LIVE		
E	318	238 / 0	0 / 0	0 / 0	80 / 0	0 / 0
C	118	105 / 0	0 / 0	0 / 0	14 / 0	0 / 0
D	35	0 / 0	0 / 0	0 / 0	35 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
E-B	-395 / 0	0.0	0.0 0.13 (4)	7.81			
A-B	0 / 23	-77.4	-77.4 0.10 (1)	10.00			
B-C	-26 / 0	-77.4	-77.4 0.47 (1)	6.25			
E-D	0 / 0	-17.5	-17.5 0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.47/1.00 (B-C-1), BC=0.13/1.00 (D-E-4),
WB=0.00/1.00 (n/a:0), SSI=0.20/1.00 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)		
MT20	618	354	1557
	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

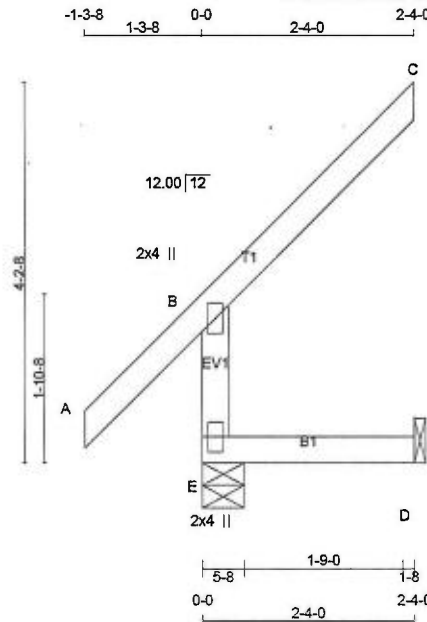
JSI GRIP= 0.23 (B) (INPUT = 0.90)
JSI METAL = 0.09 (B) (INPUT = 1.00)



September 14, 2018

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:24.4

TOTAL WEIGHT = 2 X 11 = 21 lb [M]

LUMBER
N. L. G. A. RULES
CHORDS SIZE LUMBER
E - B 2x4 DRY No.2
A - C 2x4 DRY No.2
E - D 2x4 DRY No.2

DRY, SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
E	319	0	319	0	5-8	1-8
D	19	0	21	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) D

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
E	219	178 / 0	0 / 0	0 / 0	0 / 0	41 / 0	0 / 0
D	15	0 / 0	0 / 0	0 / 0	0 / 0	15 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED HORIZ. LOAD (LC1)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (LC1)
FR-TO		FROM	TO	UNBRACED LENGTH	FR-TO		
E-B	-296 / 0	0.0	0.0	0.01 (4)	7.81		
A-B	0 / 38	-77.4	-77.4	0.11 (1)	10.00		
B-C	-67 / 0	-77.4	-77.4	0.32 (1)	6.25		
E-D	0 / 0	-17.5	-17.5	0.02 (4)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. CG

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.32/1.00 (B-C:1), BC=0.02/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.10/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

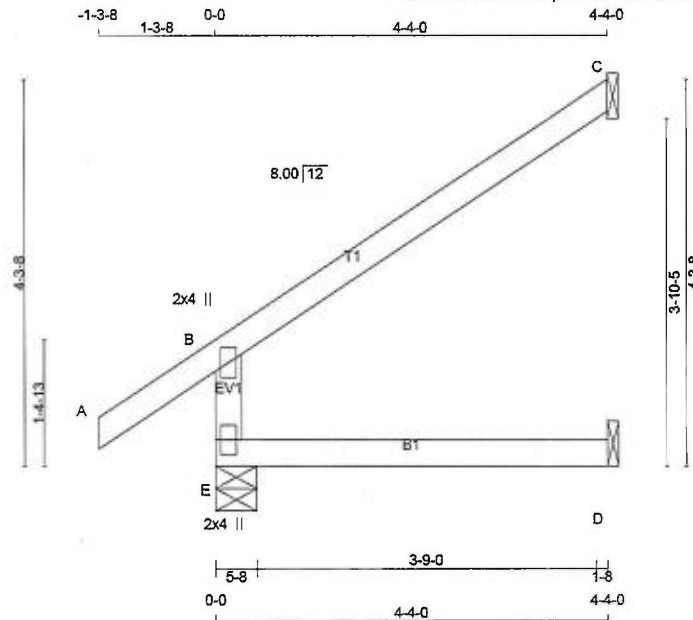
JSI GRIP= 0.23 (B) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)



September 14, 2018

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:24.5

TOTAL WEIGHT = 2 X 14 = 28 lb [M]

LUMBER
N. L. G. A. RULES
CHORDS SIZE LUMBER
E - B 2x4 DRY No.2
A - C 2x4 DRY No.2
E - D 2x4 DRY No.2
DRY: SEASONED LUMBER.

PLATES (table is in inches)
JT TYPE PLATES W LEN Y X
B TMV+p MT20 2.0 4.0
E BMV1+p MT20 2.0 4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT REQRD	
	VERT	HORZ	DOWN	HORZ	BRG IN-SX	BRG IN-SX
E	359	0	359	0	5-8	1-8
C	126	0	126	0	1-8	1-8
D	32	0	36	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
E	249	190/0	0/0	0/0	0/0	53/0	0/0
C	85	76/0	0/0	0/0	0/0	10/0	0/0
D	26	0/0	0/0	0/0	0/0	26/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS			WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 PLF	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO		FR-TO		
E-B	-316 / 0	0.0 0.0	0.06 (4)	7.81		
A-B	0 / 29	-77.4 -77.4	0.10 (1)	10.00		
B-C	-23 / 0	-77.4 -77.4	0.25 (1)	6.25		
E-D	0 / 0	-17.5 -17.5	0.07 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. CG

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL)= L/999 (0.01")

CSI: TC=0.25/1.00 (B-C:1), BC=0.07/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 616 354 1667 622 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

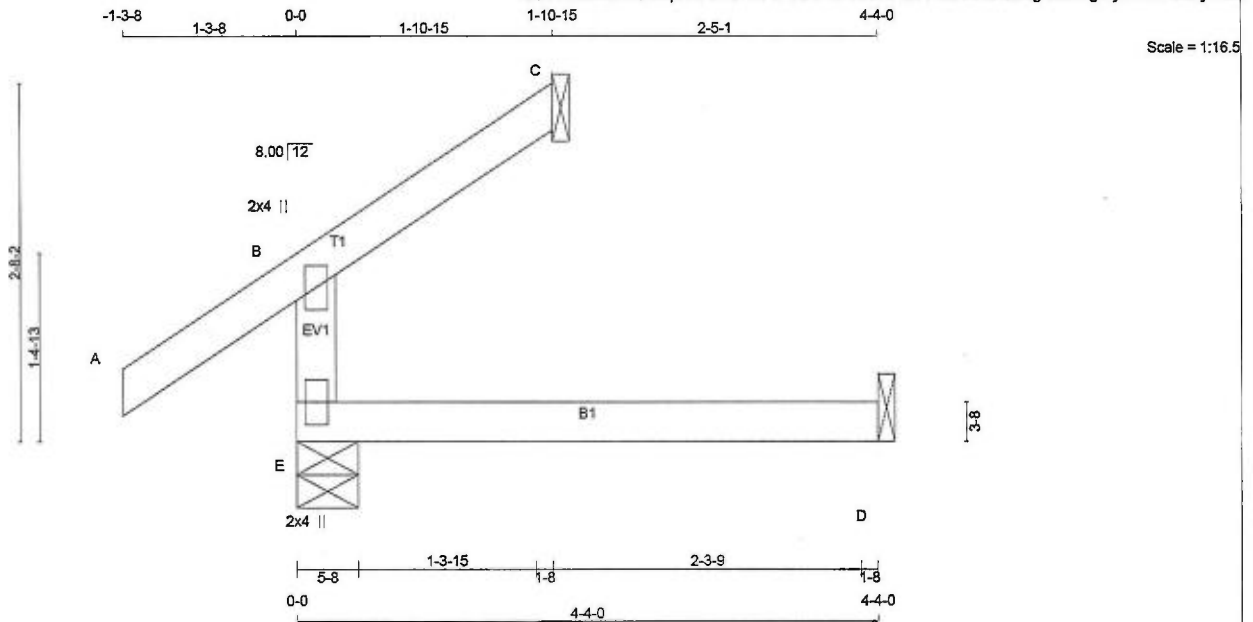
JSI GRIP= 0.20 (B) (INPUT = 0.90)
JSI METAL = 0.08 (B) (INPUT = 1.00)



September 14, 2018

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 11 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0	
E	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS, AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING

DESIGNER BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
E	248	0	248	0	5-8	1-8
C	50	0	50	0	1-8	1-8
D	32	0	38	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	173	130/0	0/0	0/0	0/0	43/0	0/0
C	35	24/0	0/0	0/0	0/0	11/0	0/0
D	26	0/-1	0/0	0/0	0/0	27/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

FR-TO	CHORDS		FACTORED		WEBS	
	MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. (LC)	MEMB.	FORCE (LBS)
E-B	-204/0	0.0	0.0	0.04 (4)	7.81	
A-B	0/29	-77.4	-77.4	0.10 (1)	10.00	
B-C	-13/4	-77.4	-77.4	0.06 (1)	6.25	
E-D	0/0	-17.5	-17.5	0.08 (4)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.01")

CSI: TC=0.10/1.00 (A-B:1), BC=0.08/1.00 (D-E:4), WS=0.00/1.00 (n/a:0), SS=0.07/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 622 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

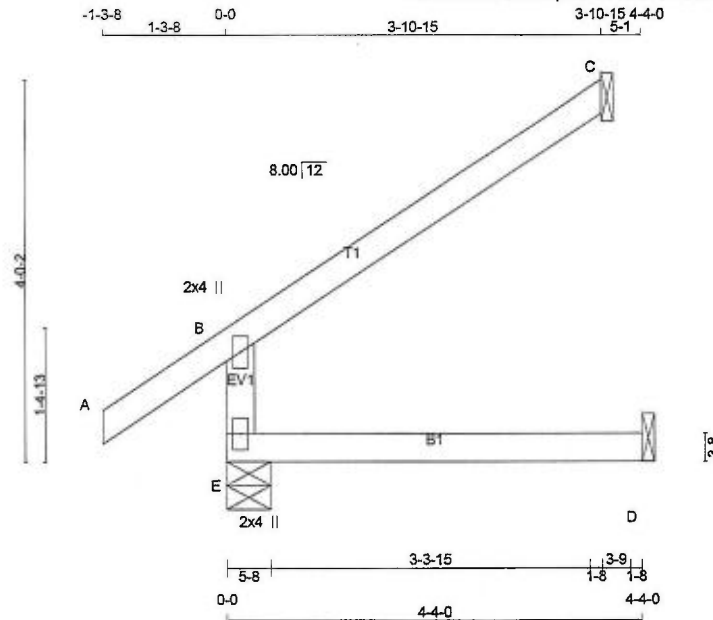
JSI GRIP= 0.13 (B) (INPUT = 0.90)
JSI METAL = 0.05 (B) (INPUT = 1.00)



September 14, 2018

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 13 lb [M]

LUMBER
N. L. G. A. RULES
CHORDS SIZE LUMBER
E - B 2x4 DRY No.2
A - C 2x4 DRY No.2
E - D 2x4 DRY No.2
DRY: SEASONED LUMBER.

PLATES (table is in inches)
JT TYPE PLATES W LEN Y X
B TMV+p MT20 2.0 4.0
E BMV1+p MT20 2.0 4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING

DESIGNER BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
VERT	HORZ	DOWN	HORZ	UPLIFT
E	339	0	339	0
C	114	0	114	0
D	32	0	36	0

SEE MITEK STANDARD DETAIL B3759H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
COMBINED							
E	235	178/0	0/0	0/0	0/0	58/0	0/0
C	77	68/0	0/0	0/0	0/0	9/0	0/0
D	26	0/0	0/0	0/0	0/0	26/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED MEMB. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. UNBRACED LENGTH	WEBS	MAX. FACTORED MEMB. FORCE (LBS)	FACTORED MAX. CSI (LC)
FR-TO		FROM	TO		FR-TO		
E-B	-295/0	0.0	0.0	0.06 (4)	7.81		
A-B	0/29	-77.4	-77.4	0.10 (1)	10.00		
B-C	-21/0	-77.4	-77.4	0.20 (1)	6.25		
E-D	0/0	-17.5	-17.5	0.07 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.20/1.00 (B-C:1), BC=0.07/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1567 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

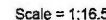
JSI GRIP= 0.19 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)



September 14, 2018

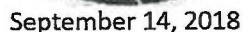
READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





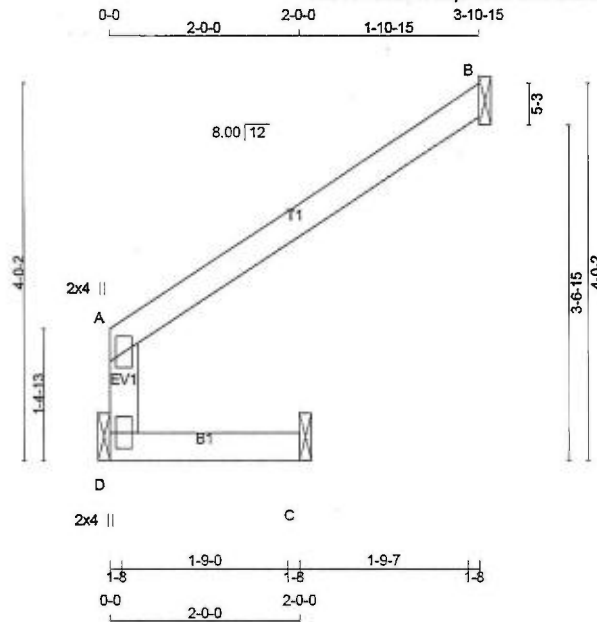
TOTAL WEIGHT = 6 lb

JSI GRIP= 0.05 (A) (INPUT = 0.90)
JSI METAL= 0.02 (A) (INPUT = 1.00)



**READ ALL NOTES ON THIS PAGE AND ON THE
ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE
IS AN INTEGRAL PART OF THIS DRAWING AS IT
CONTAINS SPECIFICATIONS AND CRITERIA USED
IN THE DESIGN OF THIS COMPONENT.**





Scale = 1:23.4

TOTAL WEIGHT = 9 lb

LUMBER
N.L.G.A. RULES
CHORDS SIZE LUMBER
D - A 2x4 DRY No.2
A - B 2x4 DRY No.2
D - C 2x4 DRY No.2
DRY: SEASONED LUMBER.

PLATES (table is in inches)
JT TYPE PLATES W LEN Y X
A TMV+p MT20 2.0 4.0
D BMV1+p MT20 2.0 4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING

JT	DESIGNER BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT REQRD	
	VERT	HORZ	DOWN	HORZ	DOWN	HORZ	BRG IN-SX	BRG IN-SX
D	152	0	152	0	152	0	1-8	1-8
B	134	0	134	0	134	0	1-8	1-8
C	52	0	52	0	52	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) D, B, C

JT	UNFACTORED REACTIONS		1ST LCASE		MAX/MIN COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	SNOW	LIVE	LIVE	PERM. LIVE			
D	106	81/0	0/0	0/0	0/0	0/0	0/0	25/0	0/0
B	91	80/0	0/0	0/0	0/0	0/0	0/0	11/0	0/0
C	37	21/0	0/0	0/0	0/0	0/0	0/0	16/0	0/0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

FR-TO	CHORDS		FACTORED		WEBS		FACTORED	
	MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	MEMB.	FORCE (LBS)	MAX. FORCE (LBS)	MAX. FORCE (LC)
D-A	-169/0	0.0	0.0	0.08 (1)	7.81	10.00		
A-B	-10/0	-77.4	-77.4	0.15 (1)	10.00			
D-C	0/0	-17.5	-17.5	0.09 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.00")
ALLOWABLE DEFL. (TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.00")

CSI: TC=0.15/1.00 (A-B:1), BC=0.09/1.00 (C-D:1), WB=0.00/1.00 (n/a:0), SSI=0.11/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)	(PLI)	(PSI)	(PLI)	(PLI)	(PLI)
MT20	618	354	1867	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

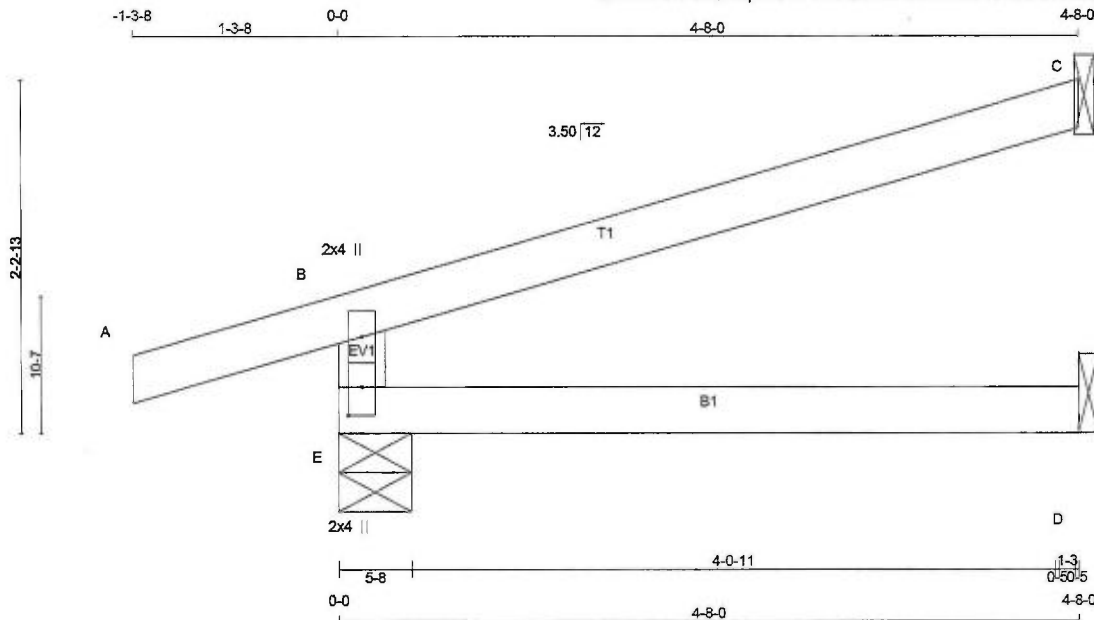
JSI GRIP= 0.11 (A) (INPUT = 0.90)
JSI METAL= 0.04 (A) (INPUT = 1.00)



September 14, 2018

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:13.9

TOTAL WEIGHT = 3 X 13 = 39 lb [M]

LUMBER
N. L. G. A. RULES
CHORDS SIZE LUMBER

E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0	2.25	1.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING

DESIGNER BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	377	0	377	0	5-8	1-8
C	135	0	135	0	1-8	1-8
D	33	0	37	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) E, C, D

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	262	198 / 0	0 / 0	0 / 0	0 / 0	64 / 0	0 / 0
C	92	82 / 0	0 / 0	0 / 0	0 / 0	11 / 0	0 / 0
D	27	0 / 0	0 / 0	0 / 0	0 / 0	27 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

FR-TO	CHORDS		FACTORED		WEBS	
	MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC) (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
E-B	-329 / 0	0.0	0.0	0.08 (4)	7.81	
A-B	0 / 14	-77.4	-77.4	0.10 (1)	10.00	
B-C	-13 / 0	-77.4	-77.4	0.29 (1)	6.25	
E-D	0 / 0	-17.5	-17.5	0.08 (4)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360 (0.19")$
CALCULATED VERT. DEFL.(LL) = $L/999 (0.00")$
ALLOWABLE DEFL.(TL) = $L/360 (0.19")$
CALCULATED VERT. DEFL.(TL) = $L/999 (0.01")$

CSI: TC=0.29/1.00 (B-C-1); BC=0.08/1.00 (D-E-4);
WB=0.00/1.00 (n/a:0); SSH=0.17/1.00 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	618	354	1587
	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

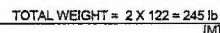
JSI GRIP= 0.18 (B) (INPUT = 0.90)
JSI METAL = 0.06 (E) (INPUT = 1.00)



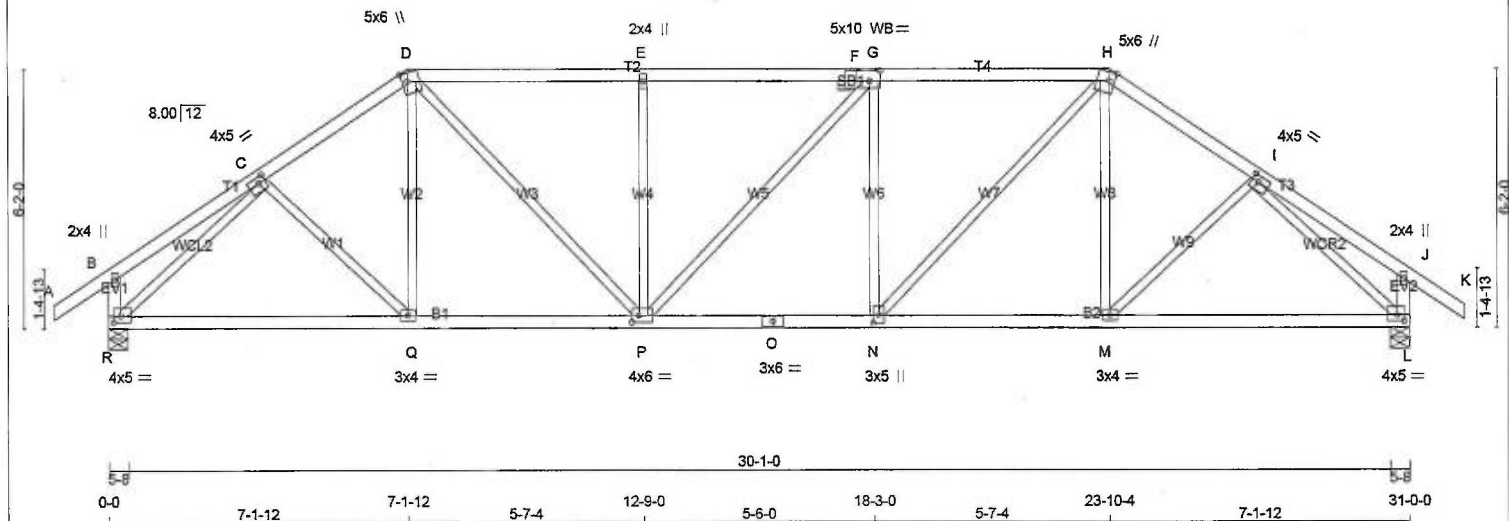
September 14, 2018

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





KOTT



TOTAL WEIGHT = 2 X 133 = 266 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE		LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
R - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - L	2x4	DRY	No.2

ALL WEBS	2x3	DRY
EXCEPT		

DRY; SEASONED LUMBER.

PLATES (table in inches)						
J	TYPE	PLATES	W	LEN	Y	X
B	TMV+P	MT20	2.0	4.0		
C	TMVW-t	MT20	4.0	5.0	1.75	2.00
D	TTWW+m	MT20	5.0	6.0	2.50	1.50
E	TMV+w	MT20	2.0	4.0		
F						
F	TSWW-I	MT20	5.0	10.0	3.00	3.00
G						
H	TTWW+m	MT20	5.0	6.0	2.50	1.50
I	TMVW-t	MT20	4.0	5.0	1.75	2.00
J	TMV+P	MT20	2.0	4.0		
K	BMVW-I	MT20	4.0	5.0	1.75	2.00
L	BMVW-I	MT20	3.0	4.0		
M	BMVW+t	MT20	3.0	5.0	2.25	1.50
N	BS-I	MT20	3.0	6.0		
O	BMVW-t	MT20	4.0	6.0	2.00	2.00
P	BMVW-I	MT20	3.0	4.0		
Q	BMVW-I	MT20	4.0	5.0	1.75	2.00

WB - INDICATES BLOCKING REQUIRED

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING

DESIGNER						
BEARINGS						
JT	FACTORED		MAXIMUM FACTORED		INPUT	REQRD
	GROSS	REACTION	GROSS	REACTION	BRG	BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX
R	1577	0	1577	0	0	5-8
L	1577	0	1577	0	0	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1104	786 / 0	0 / 0	0 / 0	0 / 0	318 / 0	0 / 0
L	1104	786 / 0	0 / 0	0 / 0	0 / 0	318 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.43 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LOCAL CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
A-B	0/29	-77.4	-77.4	0.10 (1)	C-Q	0/67	0.02 (4)
B-C	0/16	-77.4	-77.4	0.14 (1)	D-D	0/103	0.04 (4)
C-D	-1693/0	-77.4	-77.4	0.18 (1)	D-E	0/763	0.17 (1)
D-E	-1922/0	-77.4	-77.4	0.41 (4)	P-E	-464/0	0.28 (1)
E-F	-1922/0	-77.4	-77.4	0.41 (1)	P-G	-2/10	0.00 (1)
F-G	-1922/0	-77.4	-77.4	0.41 (1)	N-G	-465/0	0.28 (1)
G-H	-1939/0	-77.4	-77.4	0.41 (4)	H-H	0/765	0.17 (1)
H-I	-1693/0	-77.4	-77.4	0.18 (1)	M-H	0/103	0.04 (4)
I-J	0/16	-77.4	-77.4	0.14 (1)	M-I	0/67	0.02 (4)
J-K	0/29	-77.4	-77.4	0.10 (1)	R-C	-1879/0	0.74 (1)
R-B	-216/0	0.0	0.0	0.02 (1)	I-L	-1879/0	0.74 (1)
L-J	-216/0	0.0	0.0	0.02 (1)			
R-Q	0/1349	-17.5	-17.5	0.33 (1)	10.00		
Q-P	0/1394	-17.5	-17.5	0.33 (1)	10.00		
P-O	0/1283	-17.5	-17.5	0.38 (1)	10.00		
O-N	0/1625	-17.5	-17.5	0.36 (1)	10.00		
N-M	0/1394	-17.5	-17.5	0.33 (1)	10.00		
M-L	0/1348	-17.5	-17.5	0.32 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

SPECIALIZED LOADS:			
TOP CH.	LL	=	23.3 PSF
	DL	=	3.0 PSF
BOT CH.	LL	=	0.0 PSF
	DL	=	7.0 PSF
TOTAL LOAD		=	33.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR
SMALL BUILDING REQUIREMENTS OF PART 9, NBCC
2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL(LL) = L/999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL(TL) = L/999 (0.17")

CSI: TC=0.41/1.00 (G-H:1), BC=0.36/1.00 (N-P:1),
WB=0.74/1.00 (C-R:1), SSI=0.20/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

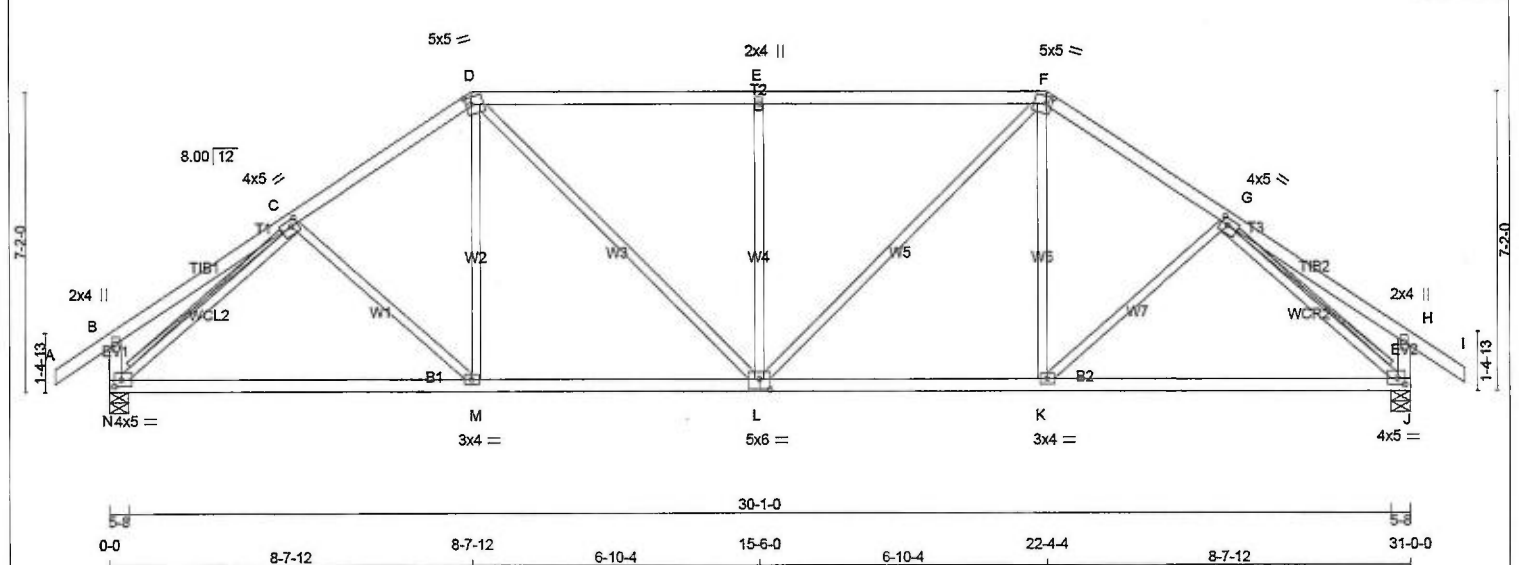
JSI GRIP= 0.87 (C) (INPUT = 0.90)
JSI METAL= 0.56 (O) (INPUT = 1.00)



September 14, 2018

**READ ALL NOTES ON THIS PAGE AND ON THE
ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE
IS AN INTEGRAL PART OF THIS DRAWING AS IT
CONTAINS SPECIFICATIONS AND CRITERIA USED
IN THE DESIGN OF THIS COMPONENT.**





LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING										TOTAL WEIGHT = 2 X 150 = 280 LBS																					
N L G A RULES				DESIGNER												[M/F]																			
CHORDS SIZE				BEARINGS																															
A - D	2x4	DRY	LUMBER	DESCR.	SPF	FACTORED	MAXIMUM FACTORED	INPUT	REQRD					DESIGN CRITERIA																					
D - F	2x4	DRY		SPF		GROSS REACTION	GROSS REACTION	BRG	BRG					SPECIFIED LOADS:																					
F - I	2x4	DRY		SPF	JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	TOP CH. LL = 23.3 PSF																						
N - B	2x4	DRY		SPF	N	1577	0	1577	0	0	5-8	1-12	DL = 3.0 PSF																						
J - H	2x4	DRY		SPF	J	1577	0	1577	0	0	5-8	1-12	BOT CH. LL = 0.0 PSF																						
N - L	2x4	DRY		SPF									DL = 7.0 PSF																						
L - J	2x4	DRY		SPF									TOTAL LOAD = 33.3 PSF																						
				UNFACTORED REACTIONS												SPACING = 24.0 IN/C																			
ALL WEBS				1ST LCASE				MAX/MIN COMPONENT REACTIONS																											
EXCEPT				JT				COMBINED				SNOW				LIVE				PERM.LIVE				WIND				DEAD				SOIL			
				N				1104				786 / 0				0 / 0				0 / 0				0 / 0				318 / 0				0 / 0			
				J				1104				786 / 0				0 / 0				0 / 0				0 / 0				318 / 0				0 / 0			
DRY: SEASONED LUMBER.																																LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM			

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	2.0	4.0		
C TMWW-t	MT20	4.0	5.0	1.75	2.00
D TTWW-m	MT20	5.0	5.0	2.00	1.50
E TMWW-w	MT20	2.0	4.0		
F TTWW-m	MT20	5.0	5.0	2.00	1.50
G TMWW-t	MT20	4.0	5.0	1.75	2.00
H TMV+p	MT20	2.0	4.0		
J BMVW1-t	MT20	4.0	5.0	1.75	2.00
K BMVW-t	MT20	3.0	4.0		
L BSWW-t	MT20	5.0	6.0	3.00	3.00
M BMWW-t	MT20	3.0	4.0		
N BMVW1-t	MT20	4.0	5.0	1.75	2.00

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.46 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT C-N, G-J

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 29	-77.4	-77.4 0.10 (1)	10.00	C-M	-55 / 44	0.03 (1)
B-C	0 / 21	-77.4	-77.4 0.22 (1)	10.00	M-D	0 / 200	0.05 (4)
C-D	-1648 / 0	-77.4	-77.4 0.21 (1)	4.98	D-L	0 / 524	0.12 (1)
D-E	-1728 / 0	-77.4	-77.4 0.53 (1)	4.46	L-E	-852 / 0	0.58 (1)
E-F	-1728 / 0	-77.4	-77.4 0.53 (1)	4.46	L-F	0 / 524	0.12 (1)
F-G	-1648 / 0	-77.4	-77.4 0.21 (1)	4.98	K-F	0 / 200	0.05 (4)
G-H	0 / 21	-77.4	-77.4 0.22 (1)	10.00	K-G	-55 / 44	0.03 (1)
H-I	0 / 29	-77.4	-77.4 0.10 (1)	10.00	N-C	-1891 / 0	0.50 (1)
N-B	-238 / 0	0.0	0.0 0.02 (1)	7.81	G-J	-1891 / 0	0.50 (1)
J-H	-238 / 0	0.0	0.0 0.02 (1)	7.81			
N-M	0 / 1393	-17.5	-17.5 0.39 (4)	10.00			
M-L	0 / 1355	-17.5	-17.5 0.40 (4)	10.00			
L-K	0 / 1355	-17.5	-17.5 0.40 (4)	10.00			
K-J	0 / 1393	-17.5	-17.5 0.39 (4)	10.00			

CS: TC=0.53/1.00 (E-F:1), BC=0.40/1.00 (L-M:4), WB=0.58/1.00 (E-L:1), SSI=0.26/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 618 354 1567 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

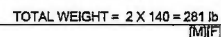
JSI GRIP= 0.88 (D) (INPUT = 0.90)

JSI METAL= 0.56 (C) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	1.75	3.00
C	TMVW-t	MT20	3.0	4.0	1.50	1.50
D	TTWVW-m	MT20	5.0	5.0	2.50	1.50
E	TMVW-t	MT20	2.0	4.0		
F	TMVW-m	MT20	5.0	5.0	2.50	1.50
G	TMVW-t	MT20	3.0	4.0	1.50	1.50
H	TMVW-t	MT20	4.0	6.0	1.75	3.00
J	BMV1+p	MT20	2.0	4.0		
K	BMVW-t	MT20	4.0	4.0	1.50	1.50
L	BSVWV-t	MT20	3.0	6.0		
N	BSVWV-t	MT20	5.0	4.0	3.00	3.00
O	BMVW-t	MT20	3.0	4.0		
P	BMVW-t	MT20	4.0	4.0	1.50	1.50
P	BMV1+p	MT20	2.0	4.0		

UNFACTORED REACTIONS							
1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1104	786 / 0	0 / 0	0 / 0	0 / 0	318 / 0	0 / 0
J	1104	786 / 0	0 / 0	0 / 0	0 / 0	318 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.79 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MAX. FACTORED		FACTORED		MAX.	MAX. FACTORED		MAX.	MAX.	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 CSI (LC)		MEMB.	FORCE (LBS)			LC1 CSI (LC)
FR-TO		FROM	TO	LENGTH	FR-TO				
A-B	0 / 29	-77.4	-77.4	0.10 (1)	10.00	O-C	-210 / 22	0.07 (1)	
B-C	-1727 / 0	-77.4	-77.4	0.30 (1)	4.79	C-N	-240 / 0	0.21 (1)	
C-D	-1585 / 0	-77.4	-77.4	0.29 (1)	4.99	N-D	0 / 252	0.08 (1)	
D-E	-1485 / 0	-77.4	-77.4	0.31 (1)	5.05	D-M	0 / 360	0.08 (1)	
E-F	-1485 / 0	-77.4	-77.4	0.31 (1)	5.05	M-E	-505 / 0	0.65 (1)	
F-G	-1650 / 0	-77.4	-77.4	0.31 (1)	4.59	M-F	0 / 360	0.08 (1)	
G-H	-1727 / 0	-77.4	-77.4	0.30 (1)	4.78	L-F	0 / 252	0.06 (1)	
H-I	0 / 29	-77.4	-77.4	0.10 (1)	10.00	L-G	-240 / 0	0.21 (1)	
P-B	-1538 / 0	0.0	0.0	0.16 (1)	6.64	K-G	-210 / 22	0.07 (1)	
J-H	-1538 / 0	0.0	0.0	0.16 (1)	6.64	B-O	0 / 1489	0.34 (1)	
						K-H	0 / 1489	0.34 (1)	
P-O	0 / 0	-17.5	-17.5	0.10 (4)	10.00				
O-N	0 / 1458	-17.5	-17.5	0.29 (1)	10.00				
N-M	0 / 1282	-17.5	-17.5	0.26 (1)	10.00				
M-L	0 / 1458	-17.5	-17.5	0.26 (1)	10.00				
L-K	0 / 1458	-17.5	-17.5	0.28 (1)	10.00				
K-J	0 / 0	-17.5	-17.5	0.10 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD	=		33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR
SMALL BUILDING REQUIREMENTS OF PART 9, NBCC
2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , CBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/999 (0.12")

CSI: TC=0.31/1.00 (D-E:1), BC=0.29/1.00 (N-O:1),
WB=0.65/1.00 (E-M:1), SSI=0.20/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

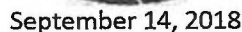
NAIL VALUES

PLATE	GRIP (DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

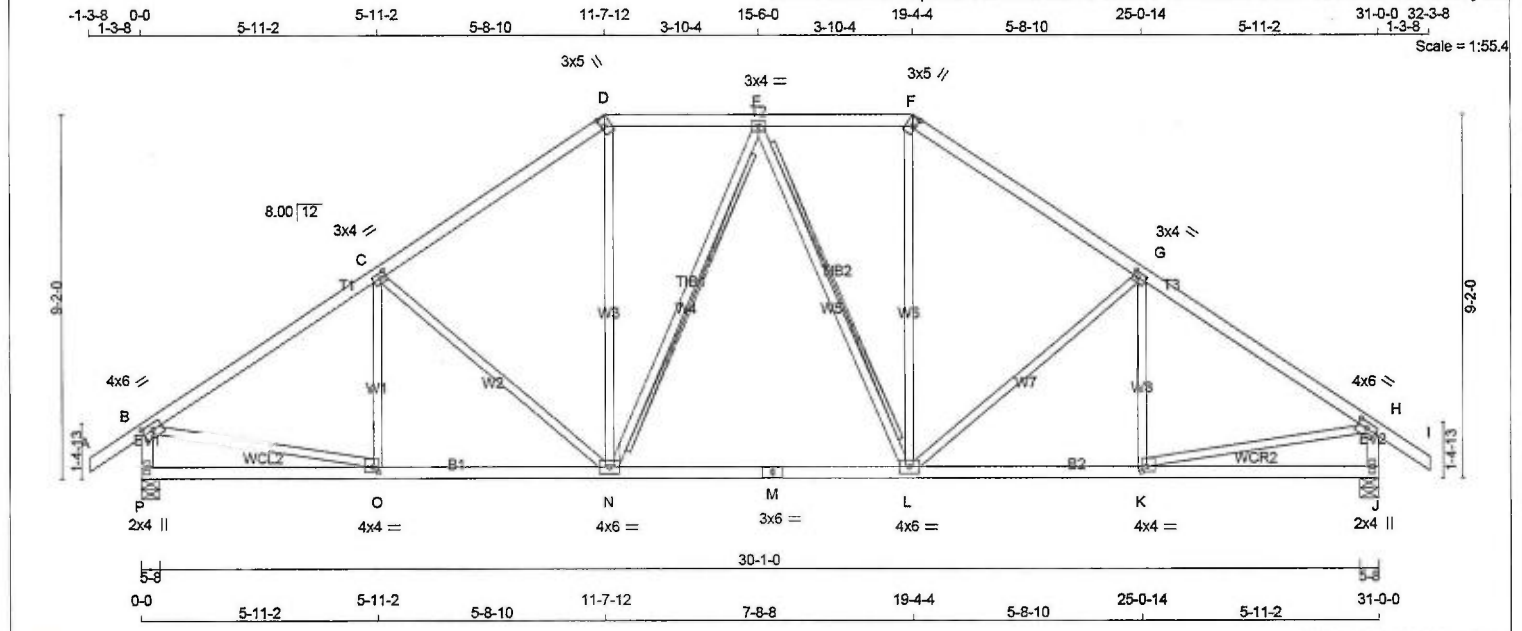
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL= 0.55 (O) (INPUT = 1.00)



**READ ALL NOTES ON THIS PAGE AND ON THE
ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE
IS AN INTEGRAL PART OF THIS DRAWING AS IT
CONTAINS SPECIFICATIONS AND CRITERIA USED
IN THE DESIGN OF THIS COMPONENT.**





TOTAL WEIGHT = 2 X 140 = 281 lb

LUMBER

N. L. G. A. RULES	SIZE	LUMBER
CHORDS		
A - D	2x4	DRY No.2
D - F	2x4	DRY No.2
F - I	2x4	DRY No.2
P - B	2x4	DRY No.2
J - H	2x4	DRY No.2
P - M	2x4	DRY No.2
M - J	2x4	DRY No.2
ALL WEBS EXCEPT	2x3	DRY No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	1.75	3.0
C	TMVW-t	MT20	3.0	4.0	1.50	1.50
D	TTW-h	MT20	3.0	5.0	2.50	1.00
E	TMVW-t	MT20	3.0	4.0		
F	TTW-h	MT20	3.0	5.0	2.50	1.00
G	TMVW-t	MT20	3.0	4.0	1.50	1.50
H	TMVW-t	MT20	4.0	6.0	1.75	3.0
J	BMV1+p	MT20	2.0	4.0		
K	BMVW-t	MT20	4.0	4.0	1.50	1.50
L	BMVW-t	MT20	4.0	6.0		
M	BS-t	MT20	3.0	6.0		
N	BMVW-t	MT20	4.0	6.0		
O	BMVW-t	MT20	4.0	4.0	1.50	1.50
P	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ UPLIFT
P	1577	0	1577	0
J	1577	0	1577	0

UNFACTORED REACTIONS

	1ST LCASE	MAX/MIN COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL
P	1104	786 / 0 0 / 0 0 / 0 0 / 0 318 / 0 0 / 0
J	1104	786 / 0 0 / 0 0 / 0 0 / 0 318 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.65 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT E-N, E-L
FASTEN T AND L-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)
FR-TO				FR-TO			
A-B	0 / 29	-77.4	-77.4 0.10 (1)	O-C	-163 / 44	0.07 (1)	10.00
B-C	-1739 / 0	-77.4	-77.4 0.41 (1)	C-N	-344 / 0	0.41 (1)	4.65
C-D	-1488 / 0	-77.4	-77.4 0.38 (1)	N-D	0 / 524	0.12 (1)	4.97
D-E	-1217 / 0	-77.4	-77.4 0.15 (1)	E-N	-193 / 0	0.15 (1)	5.65
E-F	-1217 / 0	-77.4	-77.4 0.15 (1)	F-L	-193 / 0	0.15 (1)	5.65
F-G	-1488 / 0	-77.4	-77.4 0.38 (1)	L-F	0 / 524	0.12 (1)	4.97
G-H	-1739 / 0	-77.4	-77.4 0.41 (1)	H-G	-344 / 0	0.41 (1)	4.65
H-I	0 / 29	-77.4	-77.4 0.10 (1)	I-G	-163 / 44	0.07 (1)	10.00
P-B	-1532 / 0	0.0	0.0 0.16 (1)	B-O	0 / 1496	0.34 (1)	6.65
J-H	-1532 / 0	0.0	0.0 0.16 (1)	K-H	0 / 1496	0.34 (1)	6.65
P-O	0 / 0	-17.5	-17.5 0.13 (4)				10.00
O-N	0 / 1472	-17.5	-17.5 0.33 (1)				10.00
N-M	0 / 1292	-17.5	-17.5 0.30 (1)				10.00
M-L	0 / 1292	-17.5	-17.5 0.30 (1)				10.00
L-K	0 / 1472	-17.5	-17.5 0.33 (1)				10.00
K-J	0 / 0	-17.5	-17.5 0.13 (4)				10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL) = L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/999 (0.16")

CSI: TC=0.41/1.00 (B-C-1), BC=0.33/1.00 (N-O-1), WB=0.41/1.00 (C-N-1), SSI=0.18/1.00 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PLI) (PSI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1867 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

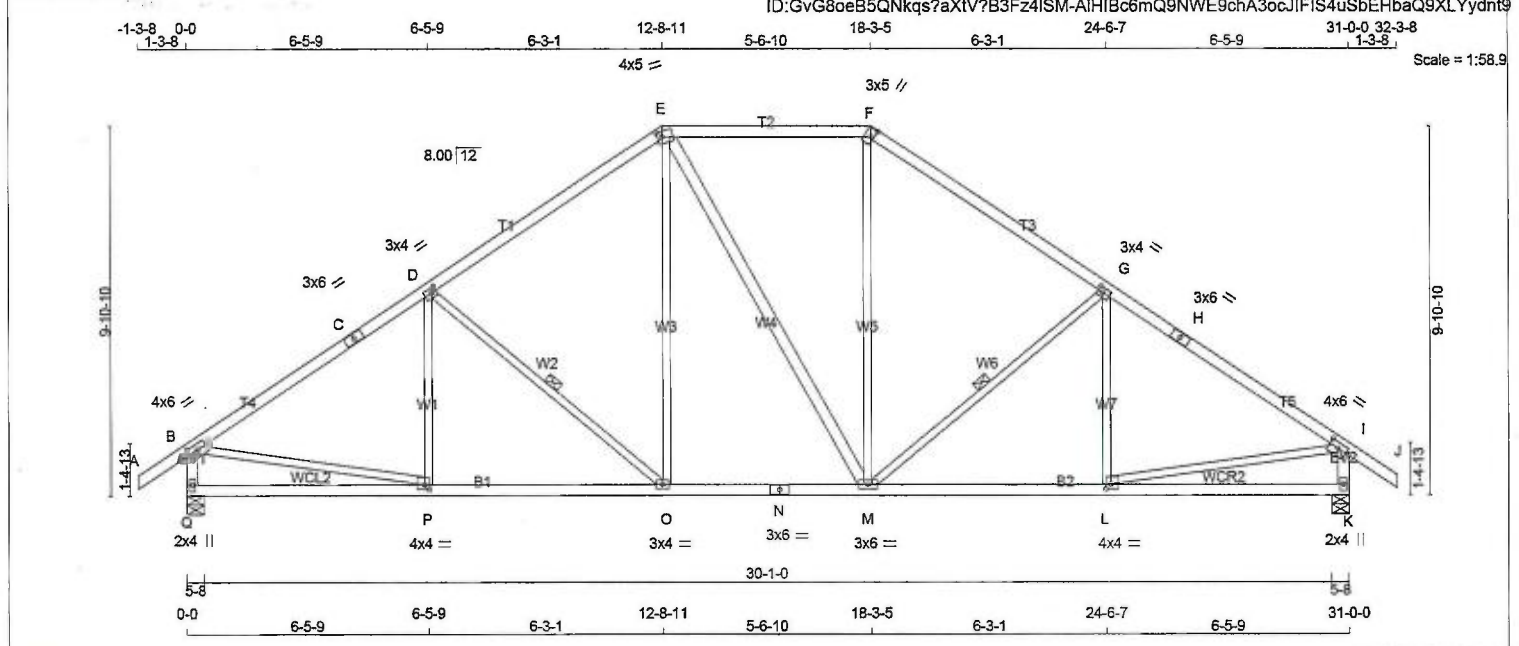
JSI GRIP= 0.87 (K) (INPUT = 0.90)
JSI METAL = 0.55 (O) (INPUT = 1.00)



September 14, 2018

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
J - K	2x4	DRY	No.2	SPF
K - L	2x4	DRY	No.2	SPF
L - M	2x4	DRY	No.2	SPF
M - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT E - M	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGNER BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
Q	1577	1577	0	0
K	1577	1577	0	0

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1104	786 / 0	0 / 0	0 / 0	0 / 0	318 / 0	0 / 0
K	1104	786 / 0	0 / 0	0 / 0	0 / 0	318 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.53 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF D-O, G-M, DBS = 20-0-0, CBF = 53 LBS.

DBS = DIAGONAL BRACE SPACING (MAX), CBF = CUMULATIVE BRACING FORCE (PER BRACE), FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	WEBS MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
FR-TO				FR-TO		
A-B	0 / 29	-77.4	0.10 (1)	P-D	-121 / 71	0.06 (1)
B-C	-1743 / 0	-77.4	0.50 (1)	D-O	-428 / 0	0.20 (1)
C-D	-1743 / 0	-77.4	0.50 (1)	O-E	0 / 374	0.08 (1)
D-E	-1420 / 0	-77.4	0.45 (1)	E-M	0 / 1	0.00 (1)
E-F	-1157 / 0	-77.4	0.33 (1)	M-F	0 / 375	0.08 (1)
F-G	-1421 / 0	-77.4	0.45 (1)	M-G	-426 / 0	0.20 (1)
G-H	-1743 / 0	-77.4	0.50 (1)	L-G	-122 / 71	0.06 (1)
H-I	-1743 / 0	-77.4	0.50 (1)	B-P	0 / 1498	0.34 (1)
I-J	0 / 29	-77.4	0.10 (1)	L-I	0 / 1498	0.34 (1)
Q-B	-1530 / 0	0.0	0.16 (1)			
K-I	-1530 / 0	0.0	0.16 (1)			
Q-P	0 / 0	-17.5	0.18 (4)			
P-O	0 / 1478	-17.5	0.33 (1)			
O-N	0 / 1156	-17.5	0.25 (1)			
N-M	0 / 1156	-17.5	0.25 (1)			
M-L	0 / 1477	-17.5	0.32 (1)			
L-K	0 / 0	-17.5	0.17 (4)			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF

BOT CH. LL = 3.0 PSF

DL = 0.0 PSF

DL = 7.0 PSF

TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014

- CSA 086-09

- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.03")

CALCULATED VERT. DEFL.(LL) = L/969 (0.06")

ALLOWABLE DEFL.(TL) = L/360 (1.03")

CALCULATED VERT. DEFL.(TL) = L/999 (0.13")

CSI: TC=0.50/1.00 (B-D:1), BC=0.33/1.00 (O-P:1), WB=0.34/1.00 (B-P:1), SSI=0.20/1.00 (B-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

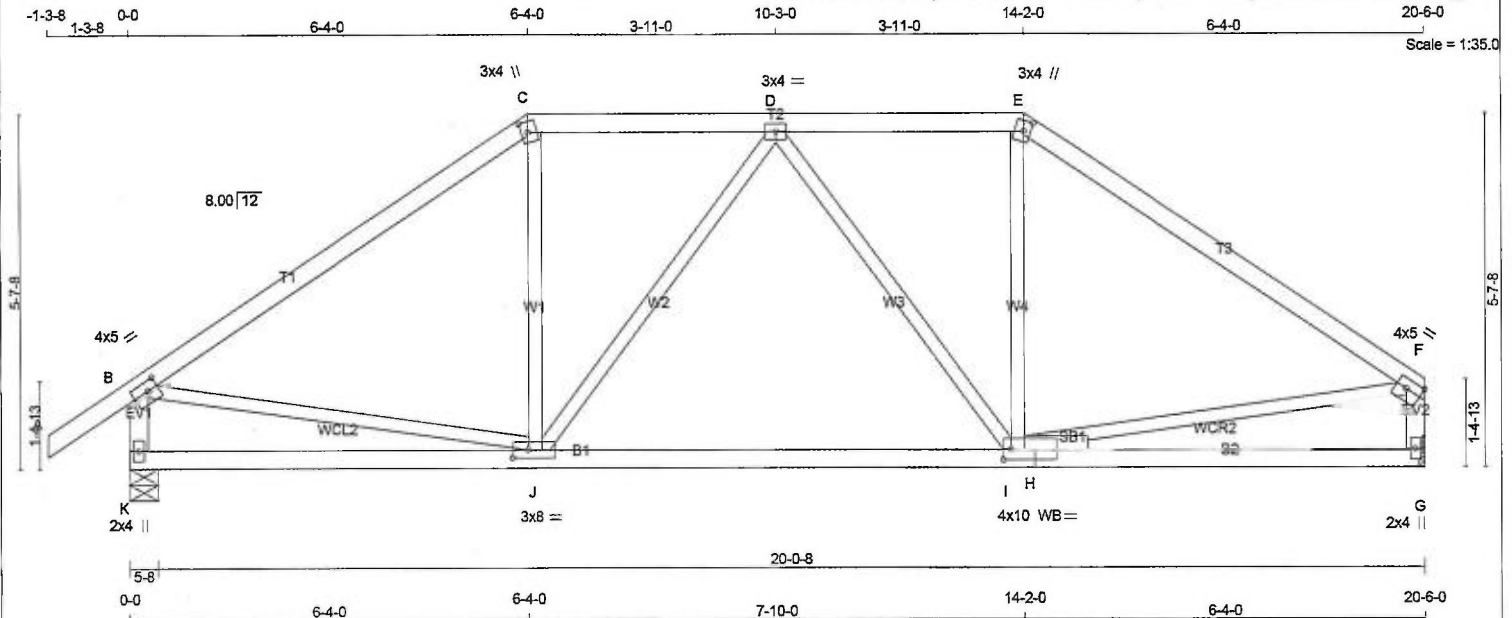
JSI GRIP= 0.89 (F) (INPUT = 0.90)

JSI METAL = 0.55 (P) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER

N.L.G.A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
K - B	2x4	DRY	No.2
G - F	2x4	DRY	No.2
K - H	2x4	DRY	No.2
H - G	2x4	DRY	No.2

ALL WEBS EXCEPT 2x3 DRY No.2

SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	5.0	1.75	2.00
C	TTW+m	MT20	3.0	4.0	2.00	1.25
D	TMVW-t	MT20	3.0	4.0		
E	TTW+m	MT20	3.0	4.0	2.00	1.25
F	TMVW-t	MT20	4.0	5.0	1.75	Edge
G	BMV1+p	MT20	2.0	4.0		
H	BSWWW-t	MT20	4.0	10.0	2.00	1.50
I	BMVWW-t	MT20	3.0	8.0	1.50	3.00
J	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

WB - INDICATES BLOCKING REQUIRED

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING

DESIGNER BEARINGS

FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT VERT	HORZ	DOWN	HORZ
K	1079	0	1079
G	972	0	972

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT G TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS
JT COMBINED	SNOW
K	755
G	682

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.73 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)
FR-TO				FR-TO			
A-B	0/29	-77.4	-77.4 0.10 (1)	J-C	0/183	0.04 (4)	0.04 (4)
B-C	-983/0	-77.4	-77.4 0.42 (1)	J-D	-225/0	0.18 (1)	0.18 (1)
C-D	-823/0	-77.4	-77.4 0.15 (1)	D-I	-225/0	0.18 (1)	0.18 (1)
D-E	-823/0	-77.4	-77.4 0.15 (1)	I-E	0/183	0.04 (4)	0.04 (4)
E-F	-983/0	-77.4	-77.4 0.42 (1)	B-J	0/829	0.19 (1)	0.19 (1)
K-B	-1034/0	0.0	0.0 0.11 (1)	I-F	0/829	0.19 (1)	0.19 (1)
G-F	-828/0	0.0	0.0 0.10 (1)				
K-J	0/0	-17.5	-17.5 0.20 (4)				
J-I	0/952	-17.5	-17.5 0.26 (4)				
I-H	0/0	-17.5	-17.5 0.20 (4)				
H-G	0/0	-17.5	-17.5 0.20 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 23.3 PSF
DL	= 3.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 33.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/999 (0.07")

CSL: TC=0.42/1.00 (E-F:1), BC=0.26/1.00 (I-J:4), WB=0.19/1.00 (F-I:1), SS=0.16/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PLI) (PLI)
(PSI) (PSI) MIN MAX MIN MAX MIN
MT20 618 354 1667 622 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.87 (C) (INPUT = 0.90)
JSI METAL= 0.35 (B) (INPUT = 1.00)



September 14, 2018

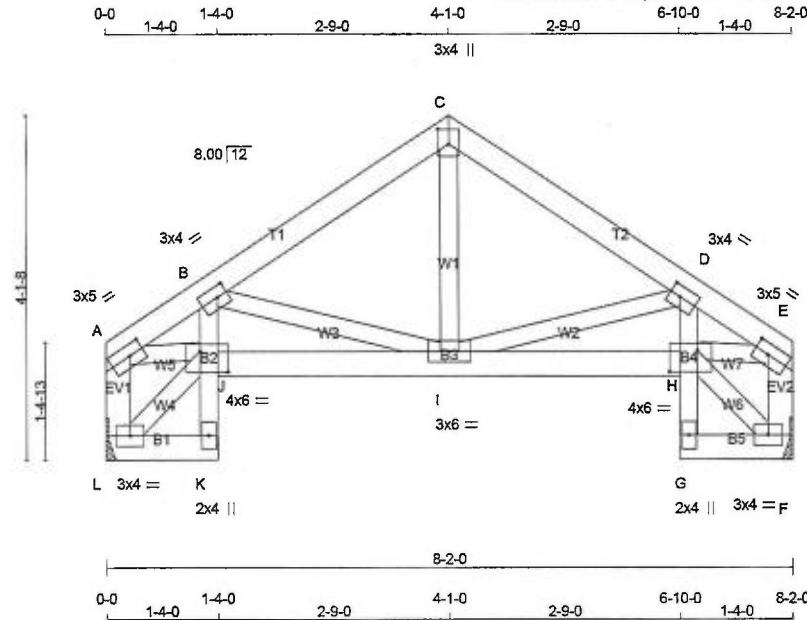
READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



Kott, Ottawa, Ontario, IM

ID:GvG8oeB5QNkqs?axiV?B3Fz4ISM-exrhOy7OBSVNsJbJmJrsWoZkUI2Kvlp4u4t_ydnt8

Version 8.210 S Mar 12 2018 MiTek Industries, Inc. Fri Sep 14 08:06:13 2018 Page 1



Scale = 1:26.3

TOTAL WEIGHT = 36 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2	
C - E	2x4	DRY	No.2	
L - A	2x4	DRY	No.2	
F - E	2x4	DRY	No.2	
L - K	2x4	DRY	No.2	
K - B	2x3	DRY	No.2	
J - H	2x4	DRY	No.2	
G - D	2x3	DRY	No.2	
G - F	2x4	DRY	No.2	

ALL WEBS 2x3 DRY No.2 EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMVW-1	MT20	3.0	5.0	1.50 2.00
B	TMVW-1	MT20	3.0	4.0	1.50 1.50
C	TTW+p	MT20	3.0	4.0	2.25 1.50
D	TMVW-1	MT20	3.0	4.0	1.50 1.50
E	TMVW-1	MT20	3.0	5.0	1.50 2.00
F	BMVW-1	MT20	3.0	4.0	
G	BMV+p	MT20	2.0	4.0	
H	BVMWV-1	MT20	4.0	6.0	3.00 4.00
I	BVMWV-1	MT20	3.0	6.0	
J	BVMWV-1	MT20	4.0	6.0	3.00 4.00
K	BMV+p	MT20	2.0	4.0	
L	BMVW-1	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

DESIGNER	FACTORED	MAXIMUM FACTORED	INPUT	REQD
BEARINGS	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
L	387	0	387	0
F	387	0	387	0

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT L, F TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN	COMPONENT REACTIONS
JT	272	190 / 0
L	272	190 / 0
F	272	190 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM	TO		FR-TO			
A-B	-632 / 0	-77.4	-77.4	0.04 (1)	6.25	I-C	0 / 186	0.04 (1)
B-C	-352 / 0	-77.4	-77.4	0.08 (1)	6.25	I-D	-278 / 0	0.05 (1)
C-D	-352 / 0	-77.4	-77.4	0.08 (1)	6.25	B-E	-278 / 0	0.05 (1)
D-E	-632 / 0	-77.4	-77.4	0.04 (1)	6.25	L-J	-14 / 0	0.00 (1)
L-A	-367 / 0	0.0	0.0	0.04 (1)	7.81	A-J	0 / 538	0.12 (1)
F-E	-367 / 0	0.0	0.0	0.04 (1)	7.81	H-F	-14 / 0	0.00 (1)
						H-E	0 / 538	0.12 (1)
L-K	0 / 11	-17.5	-17.5	0.01 (4)	10.00			
K-J	0 / 12	0.0	0.0	0.03 (1)	10.00			
J-B	0 / 55	0.0	0.0	0.04 (1)	10.00			
J-I	0 / 553	-17.5	-17.5	0.11 (1)	10.00			
I-H	0 / 553	-17.5	-17.5	0.11 (1)	10.00			
G-H	0 / 12	0.0	0.0	0.03 (1)	10.00			
H-D	0 / 55	0.0	0.0	0.04 (1)	10.00			
G-F	0 / 11	-17.5	-17.5	0.01 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.27")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.27")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.08/1.00 (B-C:1), BC=0.11/1.00 (I-J:1),
WB=0.12/1.00 (A-J:1), SSI=0.08/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

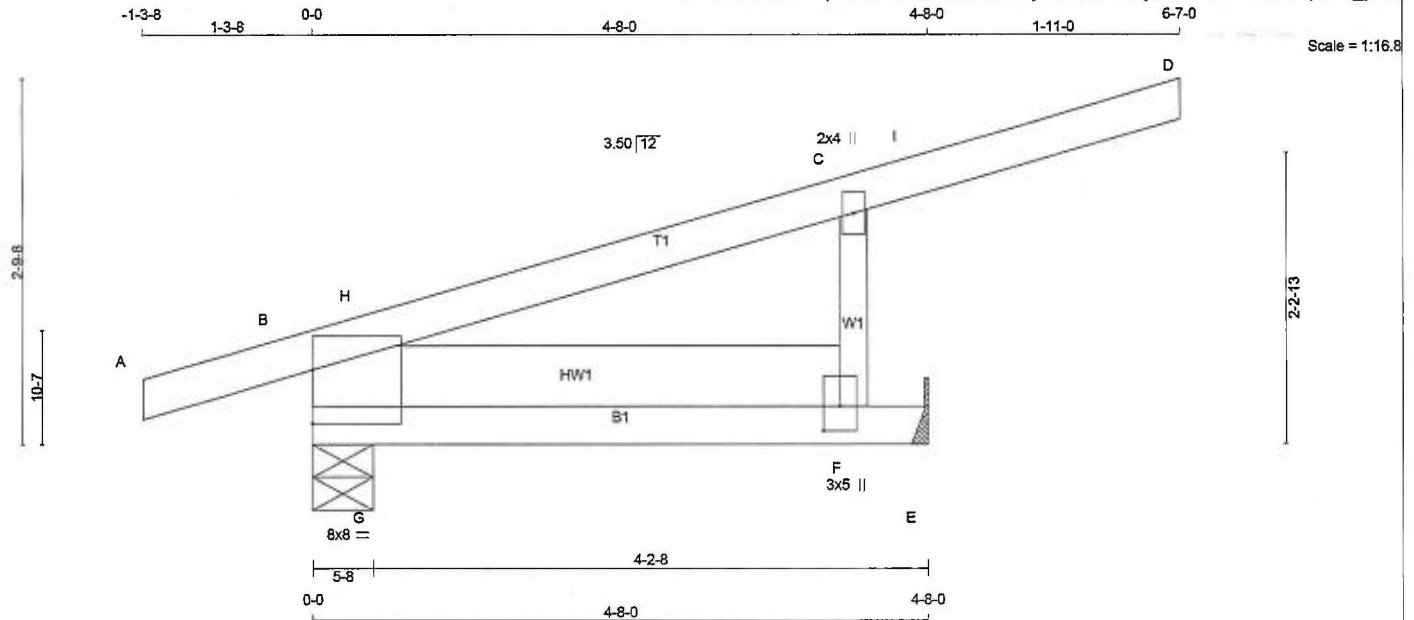
JSI GRIP= 0.64 (E) (INPUT = 0.90)
JSI METAL= 0.20 (E) (INPUT = 1.00)



September 14, 2018

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER
N.L.G.A. RULES
CHORDS SIZE LUMBER
A - D 2x4 DRY No.2
B - E 2x4 DRY No.2
REINFORCING MEMBERS
HW1 2x6 DRY No.2
ALL WEBS 2x3 DRY
DRY: SEASONED LUMBER.

PLATES (table is in inches)
JT TYPE PLATES W LEN Y X
B TMBMW1-I MT20 8.0 8.0 5.00
C TMW+W MT20 2.0 4.0
F BMWW+1 MT20 3.0 5.0 2.25 1.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING

DESIGNER BEARINGS

FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT VERT HORZ	DOWN HORZ UPLIFT	IN-SX	IN-SX
B 347 0	347 0	5-8	1-8
E 350 0	350 0	MECHANICAL	

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT E TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT COMBINED						
B 241	184 / 0	0 / 0	0 / 0	0 / 0	58 / 0	0 / 0
E 243	186 / 0	0 / 0	0 / 0	0 / 0	57 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	-77.4	-77.4	0.09 (1)	10.00	F-C	-311 / 0	0.05 (1)
B-H	-548 / 0	-77.4	0.12 (1)	6.25	G-H	0 / 293	0.00 (1)
H-C	-14 / 0	-77.4	0.12 (1)	6.25	H-F	-385 / 0	0.05 (1)
C-I	-28 / 0	-77.4	0.35 (1)	6.25			
I-D	-28 / 0	-77.4	0.35 (1)	6.25			
B-G	0 / 385	-17.5	0.18 (1)	10.00			
G-F	0 / 385	-17.5	0.33 (1)	10.00			
F-E	0 / 0	-17.5	0.26 (1)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.35/1.00 (C-D:1), BC=0.33/1.00 (F-G:1),
WB=0.05/1.00 (F-H:1), SSI=0.27/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 622 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.51 (F) (INPUT = 0.90)
JSI METAL = 0.17 (F) (INPUT = 1.00)



September 14, 2018

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



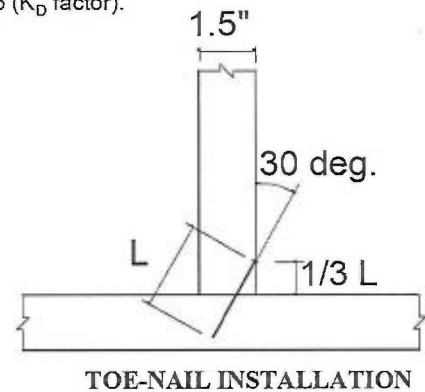
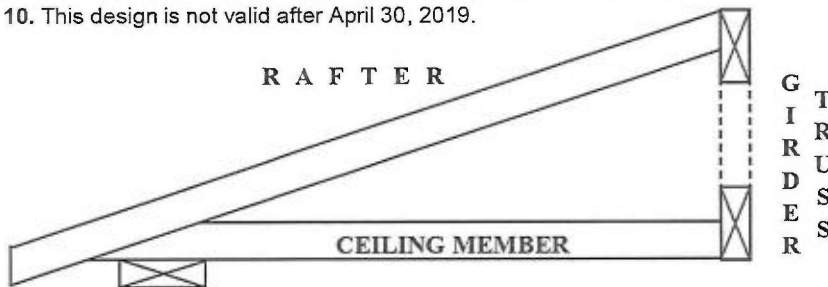
BEARING ANCHORAGE BY TOE-NAILS FOR LATERAL CAPACITY

B37579H1

NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL LATERAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	132	147
	3.25	0.144	132	147
	3.50	0.160	159	177

NOTES:

1. Rafter and ceiling members may be anchored to top and bottom chords of girder truss by toe-nailing rafter and ceiling members to girder chords provided the reaction does not exceed the lateral capacities in the table. Hangers (specified by others) are required for reactions higher than the maximum toe-nail capacity. Reactions are based on factored loads.
2. Toe nail capacities shown in the table are for one toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-09, section 10.9.4.1.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in tables below.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See next page for nailing on bearing plate).
7. For loads due to wind the nail lateral capacity in this table may be multiplied by 1.15 (K_D factor).
8. Lumber must be dry (< 19% moisture content) at the time of nail installation.
9. Nail values in this table comply with CSA O86-09, section 10.9.4
10. This design is not valid after April 30, 2019.



Nail type	Common wire	Common spiral	Common wire
Nail dia. (in)	0.160	0.152	0.144
	(3.5" nail)		(3" and 3.25" nail)
LUMBER SIZE	MAXIMUM NUMBER OF TOE-NAILS		
2X4 SPF	2	2	3
2X4 D. Fir	2	2	2
2X6 SPF	4	4	4
2X6 D. Fir	3	3	3



MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

PEO
Certificate No. 10889485



April 26, 2017

BEARING ANCHORAGE BY TOE-NAILS FOR WIND LOADING

B37579H2

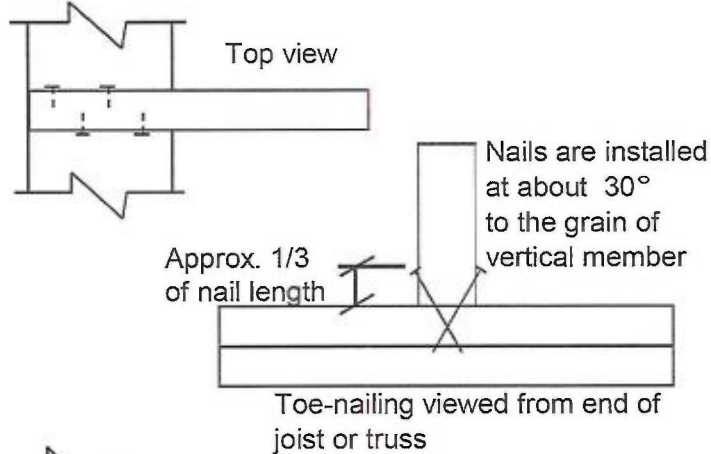
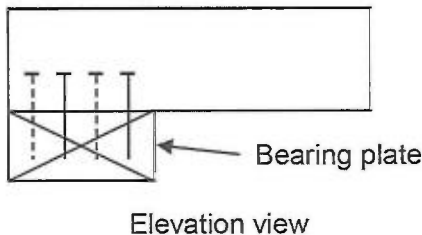
NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL WITHDRAWAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON	3.00	0.144	30	42
	3.25	0.144	32	45
	3.50	0.160	38	52

Note: If using truss with D. Fir lumber and S-P-F bearing plate, use values in table for S-P-F.

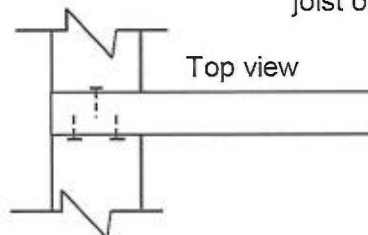
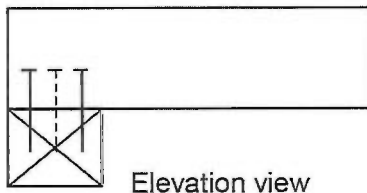
NOTES:

1. Truss chord, rafter, or ceiling members may be anchored to bearing plate by toe-nails, provided that the actual factored uplift force due to **wind or earthquake** load does not exceed the withdrawal capacities in the table. Hangers (specified by others) are required for uplift forces that are higher than the maximum toe-nail withdrawal capacity.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-09, section 10.9.5.2.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in table above.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See drawing on detail B37579H1).
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-09, section 10.9.5
9. This design is not valid after April 30, 2019

Toe-nailing on 2x6 Bearing Plate



Toe-nailing on 2x4 Bearing Plate



MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

PEO
Certificate No. 10889485



April 26, 2017