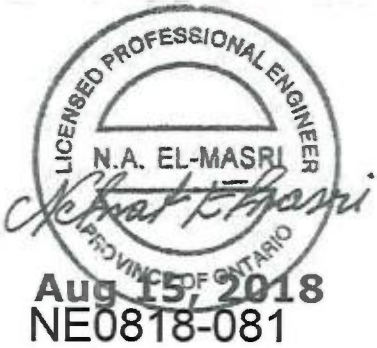




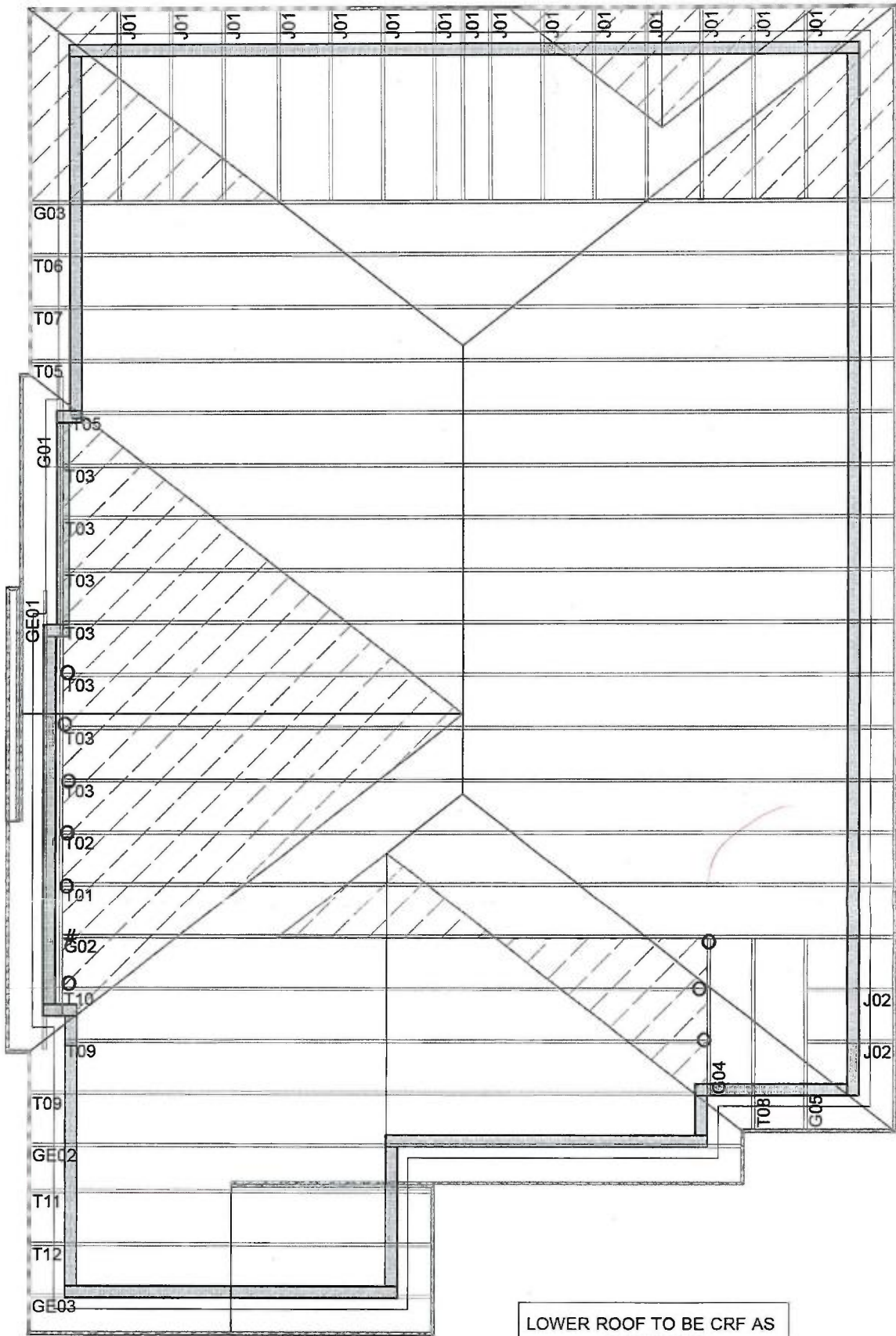
THIS CERTIFICATION IS FOR THE ENGINEERING REVIEW OF STRUCTURAL COMPONENTS SHOWN ON THIS DRAWING AND IS LIMITED TO THE COMPONENTS SUPPLIED BY KOTT LUMBER FOR THE LOADS AND CONDITIONS SHOWN ON THE CALCULATION PAGE OF EACH COMPONENT. THE PLACEMENT OF THESE COMPONENTS ON THIS LAYOUT IS SHOWN FOR REFERENCE ONLY. ROOF TRUSSES MUST BE INSTALLED ACCORDING TO MANUFACTURER'S GUIDELINES AT 24" C/C UNLESS NOTED OTHERWISE. CONVENTIONAL FRAMING SHOWN ON THE LAYOUT MUST BE DESIGNED AND APPROVED BY THE PROJECT ENGINEER. TEMPORARY AND PERMANENT BRACINGS OF THE ROOF AND BUILDING ARE THE RESPONSIBILITY OF THE PROJECT ENGINEER AND TO CONFORM TO BCSI GUIDELINES.

PORTICO FLAT ROOF
BY OTHERS

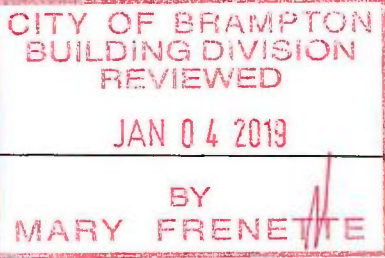
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Layout based on plans provided by
REGION DESIGN INC.
Dated MAY 2018



LOWER ROOF TO BE CRF AS
PER PLANS

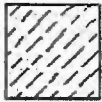


All work shall conform to the Ontario
Building Code O. Reg. 332/12 as amended
For conventional framing roof ceilings
framing shall conform to OBC.9.23

18-411414.000-00 PR

ROOF TRUSS LAYOUT

LOT 3



CONVENTIONAL
FRAMING BY OTHERS

SIZE AND LOCATION OF CONVENTIONAL FRAMING IS APPROXIMATE. ALL AREAS MAY NOT BE SHOWN. REFER TO ARCHITECTURAL PLANS FOR DETAILS.

ALL CONVENTIONAL FRAMING TO CONFORM WITH PART 9 OF THE O.B.C RAFTER THAT CROSS MEET OVER TRUSSES TO BE 2X4 SPF @ 24" O/C WITH 2X4 VERT POST TO THE TRUSS BELOW EA CROSS PT. VERT POS. THAN 6' TO HAVE A LATERAL BRACING SO THAT THE DISTANCE BETEN PT & VERT ROWS OF BRACING DOES NOT EXCEED 6'

HANGER LEGEND

X - LUS24
- HGUS26
O - LJS26DS
% - HGUS26-2

DESIGN CRITERIA
SPECIFIED LOADS:
TOP CH. LL = 23.3 psf
DL = 3.0 psf
BOT CH. LL = 0.0 psf
DL = 7.0 psf
TOTAL LOAD = 33.3 psf

SPACING @ 24" CENTERS,
THIS TRUSS SYSTEM IS DESIGN FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH PART 9 OF OBC 2012,
BCBC 2012, ABC 2012, CSA 086-09, TPIC 2011

ALLOWABLE DEFLECTION OF EACH COMPONENT = L/360

Model:	MILLWOOD 12 EL 2
Customer:	GREENPARK
Project:	MINNISALE
Location:	BRAMPTON
Drawn By:	DM
Date:	6/28/2018

ROOF TRUSSES

ENGINEERING NOTE PAGE (ENP-1)
PLEASE READ PRIOR TO INSTALLATION

NE0818-081

GREENPARK - MINNISALE HOMES -

MILLWOOD 12 ELE 2 - LOT 3

RESPONSIBILITIES

THIS DESIGN IS FOR AN INDIVIDUAL BUILDING COMPONENT AND HAS BEEN BASED ON INFORMATION PROVIDED BY THE DESIGN OFFICE OF KOTT LUMBER. THE UNDERSIGNED ENGINEER DISCLAIMS ANY RESPONSIBILITY FOR DAMAGES AS A RESULT OF FAULTY OR INCORRECT INFORMATION, SPECIFICATION AND/OR DESIGNS FURNISHED TO THE ENGINEER. THE UNDERSIGNED ENGINEER IS ONLY RESPONSIBLE FOR THE STRUCTURAL INTEGRITY OF THIS BUILDING COMPONENT FOR THE CONDITIONS AND LOADS SHOWN ON THIS DRAWING. THE STRUCTURAL INTEGRITY OF THE BUILDING AND THE VERIFICATION OF THE DIMENSIONS AND THE DESIGN LOADS USED ARE THE RESPONSIBILITY OF THE BUILDING DESIGNER.

TRUSSES ARE DESIGNED IN CONFORMANCE WITH THE RELEVANT SECTIONS OF THE NATIONAL BUILDING CODE OF CANADA OR THE CANADIAN CODE FOR FARM BUILDINGS, WHICHEVER APPLIES TO THE BUILDING TYPE INDICATED ON THE DRAWING

IT IS THE RESPONSIBILITY OF KOTT LUMBER TO ENSURE THAT TRUSSES ARE MANUFACTURED IN CONFORMANCE WITH THESE DESIGNS AND WITH THE SPECIFICATIONS OUTLINED BELOW. THE UNDERSIGNED ENGINEER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

USE AND OCCUPANCY

- The building is of the type indicated on the drawing

LOADING

- The truss loading intensity and distribution as well as load transfer mechanism is that indicated on the drawing
- No buildings, trees, parapets or other projections higher than the roof for which the trusses are used are located within a distance less than ten (10) times the difference in height, or five metres (16 ft) whichever is greater, unless the drawing indicates that the snow drifting has been taken into account

HANDLING, INSTALLATION AND BRACING

- The trusses must be handled and installed by a qualified professional as per the supplied document titled *Information for Truss Installers* and the BCSI-B1 and BCSI-B3 Summary Sheets
- The compression chords are laterally braced by continuous rigid diaphragm sheathing or as specified on the drawing
- Temporary and permanent bracing must be installed as indicated on the truss drawing and according to the BCSI-B1 and BCSI-B3 Summary Sheets. Bracing for the lateral stability of the truss is to be provided by the building designer
- It is recommended that a Professional Engineer's advice be obtained for the bracing of trusses spanning more than 12.37m (40'-7")

SUPPORTS

- The trusses are to be supported at the bearing points indicated and anchored to the supports where considered necessary by the designer of the overall structure
- Bearing sizes shown are the minimum required to prevent crushing of the truss members and do not necessarily take into account stability of the overall building structure
- Elevation of bearings must be carefully checked and shimmed to alignment for solid bearings
- Adequate wood truss bearing is the responsibility of the building designer.

DIMENSIONS

- Geometry of the truss and dimensions indicated on the drawing are identical to those of the installed truss.



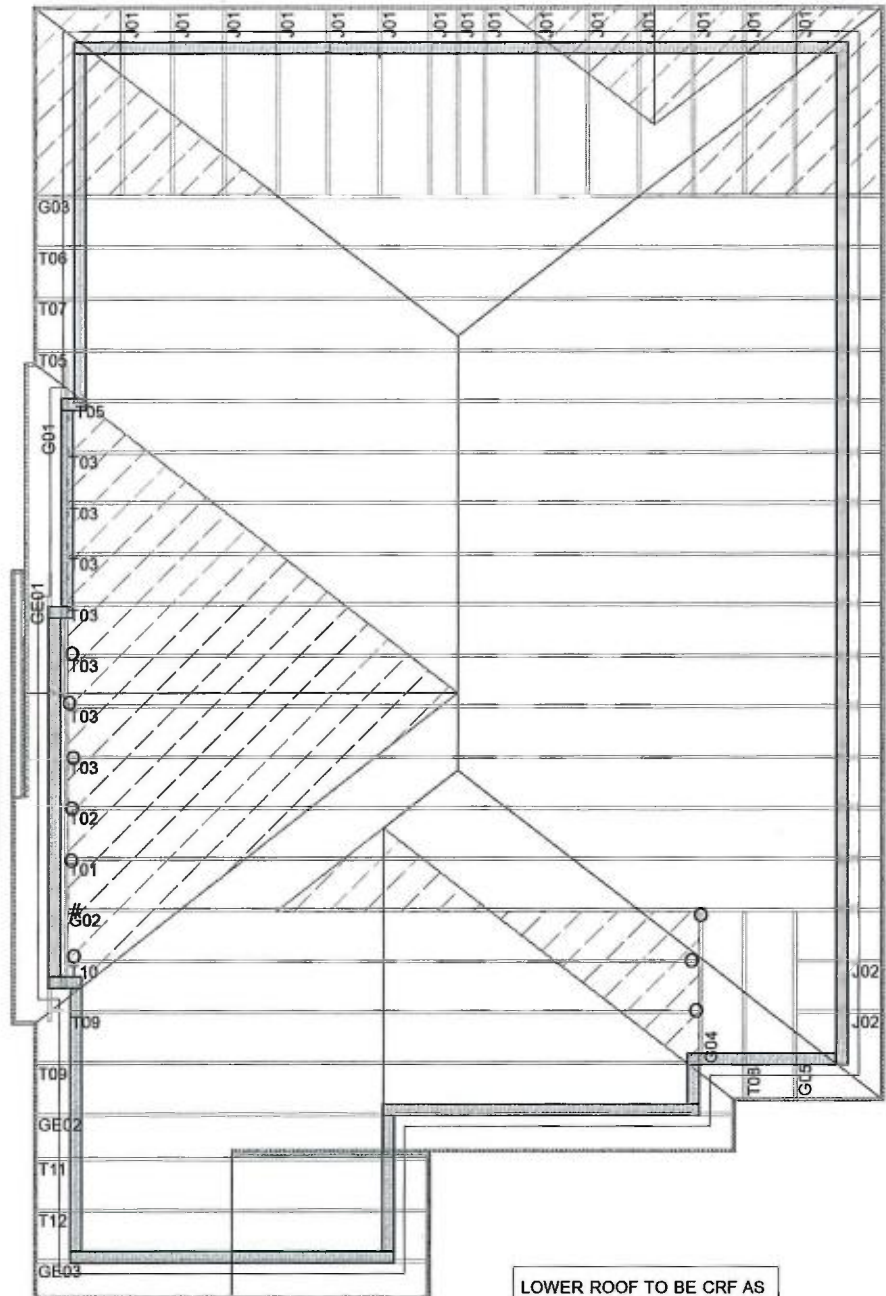
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PORTICO FLAT ROOF
BY OTHERS

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



Layout based on plans provided by
REGION DESIGN INC.
Dated MAY 2018



LOWER ROOF TO BE CRF AS
PER PLANS

ROOF TRUSS LAYOUT



CONVENTIONAL
FRAMING BY OTHERS

HANGER LEGEND

X - LUS24
- HGUS26

O - LJS26DS
% - HGUS26-2

DESIGN CRITERIA
SPECIFIED LOADS:
TOP CH. LL = 23.3 psf
DL = 3.0 psf
BOT CH. LL = 0.0 psf
DL = 7.0 psf
TOTAL LOAD = 33.3 psf

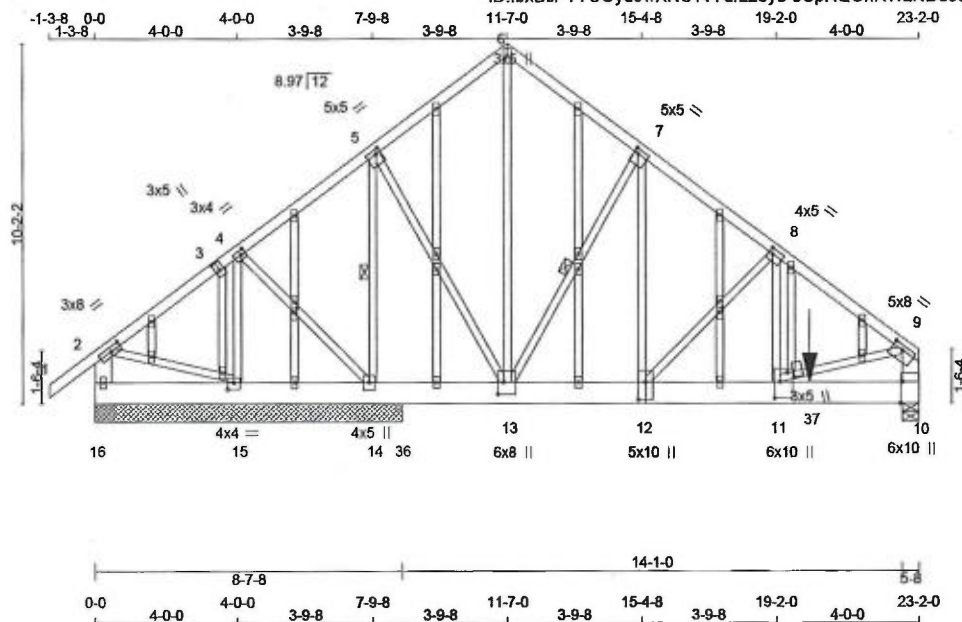
SPACING @ 24" CENTERS,
THIS TRUSS SYSTEM IS DESIGN FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH PART 9 OF CBC 2012,
BCBC 2012, ABC 2012, CSA 688-09, TFC 2011
ALLOWABLE DEFLECTION OF EACH COMPONENT = L/360

SIZE AND LOCATION OF CONVENTIONAL FRAMING IS APPROXIMATE. ALL AREAS MAY NOT BE SHOWN. REFER TO ARCHITECTURAL PLANS FOR DETAILS.

ALL CONVENTIONAL FRAMING TO CONFORM WITH PART 9 OF THE O.B.C RAFTER THAT CROSS MEET OVER TRUSSES TO BE 2X4 SPF @ 24" O/C WITH 2X4 VERT POST TO THE TRUSS BELOW EA CROSS PT. VERT POS. THAN 6" TO HAVE A LATERAL BRACING SO THAT THE DISTANCE BETEN PT & VERT ROWS OF BRACING DOES NOT EXCEED 6'

Model:	MILLWOOD 12 EL 2
Customer:	GREENPARK
Project:	MINNISALE
Location:	BRAMPTON
Drawn By:	DM
Date:	6/28/2018



TOTAL WEIGHT = 2 X 170 = 341 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
1 - 3	2x4	DRY	No.2	SPF
3 - 6	2x4	DRY	No.2	SPF
6 - 9	2x4	DRY	No.2	SPF
16 - 2	2x6	DRY	No.2	SPF
10 - 9	2x6	DRY	No.2	SPF
16 - 12	2x8	DRY	No.2	SPF
12 - 10	2x8	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
 EXCEPT

ALL GABLE WEBS 2x3 DRY No.2 SPF
 DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2'-0" OC.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
1-3	12	TOP
3-6	12	TOP
6-9	12	TOP
16-2	12	TOP
10-9	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
16-12	8	SIDE(335.0)
12-10	8	SIDE(335.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	VERT	GROSS REACTION	GROSS REACTION	DOWN	HORZ	UPLIFT	IN-SX	BRG	BRG
16	460	0	460	0	0	8-7-8	8-7-8		
14	6289	0	6289	0	0	8-7-8	8-7-8		
15	-129	0	0	0	0	-129	8-7-8		
10	7319	0	7319	0	0	5-8	5-8		

PROVIDE ANCHORAGE AT BEARING JOINT 15 FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
16	318	250 / 0	0 / 0	0 / 0	0 / 0	68 / 0	0 / 0
14	4414	3086 / 0	0 / 0	0 / 0	0 / 0	1329 / 0	0 / 0
15	-88	0 / -77	0 / 0	0 / 0	0 / 0	0 / -11	0 / 0
10	5136	3597 / 0	0 / 0	0 / 0	0 / 0	1540 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 16, 14, 15, 10
 BEARING SIZE FACTOR = 1.15 AT JNT(S) 10 (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.53 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF 7-13. DBS = 8-0-0. CBF = 188 LBS.
 1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF 5-14. DBS = 6-0-0. CBF = 162 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) TO EACH PLY USING (0.122"x3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX.		WEBS		MAX.		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	CS1 (LC)	UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX	CS1 (LC)	MEMB.	FORCE (LBS)
FR-TO		FROM TO				FR-TO				FR-TO	
1-2	0 / 32	-77.4	-77.4	0.06 (1)	10.00	13-6	0 / 2557	0.32 (1)		13-6	0 / 2557
2-3	-147 / 0	-77.4	-77.4	0.11 (1)	6.25	13-7	-3765 / 0	0.78 (1)		13-7	-3765 / 0
3-4	-147 / 0	-77.4	-77.4	0.11 (1)	6.25	12-7	0 / 4323	0.54 (1)		12-7	0 / 4323
4-5	-130 / 0	-77.4	-77.4	0.11 (1)	6.25	12-8	-2718 / 0	0.68 (1)		12-8	-2718 / 0
5-6	-2354 / 0	-77.4	-77.4	0.10 (1)	5.76	11-8	0 / 3072	0.38 (1)		11-8	0 / 3072
6-7	-2355 / 0	-77.4	-77.4	0.10 (1)	5.76	5-13	0 / 3675	0.45 (1)		5-13	0 / 3675
7-8	-4605 / 0	-77.4	-77.4	0.16 (1)	4.36	14-5	-4846 / 0	0.79 (1)		14-5	-4846 / 0
8-9	-6932 / 0	-77.4	-77.4	0.32 (1)	3.53	4-14	-58 / 0	0.01 (1)		4-14	-58 / 0
16-2	-361 / 0	0.0	0.0	0.01 (1)	7.81	15-4	-284 / 0	0.04 (1)		15-4	-284 / 0
10-9	-5739 / 0	0.0	0.0	0.21 (1)	6.15	2-15	0 / 140	0.02 (1)		2-15	0 / 140
						11-9	0 / 5741	0.71 (1)		11-9	0 / 5741
16-15	0 / 0	-17.5	-17.5	0.06 (1)	10.00						
15-14	0 / 136	-17.5	-17.5	0.30 (1)	10.00						
14-36	0 / 96	-17.5	-17.5	0.30 (1)	10.00						
36-13	0 / 96	-687.5	-687.5	0.30 (1)	10.00						
13-12	0 / 3695	-687.5	-687.5	0.40 (1)	10.00						
12-11	0 / 5557	-687.5	-687.5	0.53 (1)	10.00						
11-37	0 / 0	-687.5	-687.5	0.45 (1)	10.00						
37-10	0 / 0	-687.5	-687.5	0.45 (1)	10.00						

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
37	20-14	-1865	-1865		BACK	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
 GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
 LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF
 DL = 3.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder
 START DISTANCE = 8'-7-0
 START SPAN CARRIED = 30'-8-8
 END DISTANCE = 23'-2-0
 END SPAN CARRIED = 30'-8-8
 END WALL WIDTH = 5'-8
 APPLIED TO FRONT SIDE OF BOTTOM CHORD.
 - ADDTL LOADS BASED ON 55% OF GSL.

*** NON STANDARD GIRDER ***

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.
 THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.51")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
 ALLOWABLE DEFL.(TL)= L/360 (0.51")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.14")

CSI: TC=0.32/1.00 (8-9-1), BC=0.53/1.00 (11-12-1),
 WB=0.79/1.00 (5-14-1), SSI=0.89/1.00 (10-11-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

CONTINUED ON PAGE 2



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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
NE0818-081	G01	1	2	GREENPARK - MINNISALE HOMES MILLWOOD 12 ELE 2	PAGE 4 OF 30

Version 8.210 S Mar 12 2018 MiTek Industries, Inc. Wed Aug 15 16:46:20 2018 Page 2
ID:lbxBcPTYUOyd9wXNUTvYurZ28ye-oUpRQOhRWzNDde5di101t1?CNGCJj3TQMj5gzxynZ3X

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-I	MT20	3.0	8.0	1.50	3.50
3	TSW+I	MT20	3.0	5.0	3.00	1.50
4	TMWW-I	MT20	3.0	4.0	1.50	1.50
5	TMWW-I	MT20	5.0	5.0	1.50	1.50
6	TTW+p	MT20	3.0	5.0		
7	TMWW-I	MT20	5.0	5.0	1.50	1.25
8	TMWW-I	MT20	4.0	5.0	1.75	1.25
9	TMVW-I	MT20	5.0	8.0	1.50	3.50
10	BMV1+t	MT20	6.0	10.0	Edge	0.50
11	BMWW+t	MT20	6.0	10.0	5.50	1.75
12	BSWW+I	MT20	5.0	10.0	6.00	2.50
13	BMWWW+t	MT20	6.0	8.0	4.00	2.00
14	BMWW1+t	MT20	4.0	5.0		
15	BMWW1-t	MT20	4.0	4.0	2.25	2.00
16	BMV1+p	MT20	2.0	4.0		
17, 17, 18, 19, 20, 21, 22, 23, 24, 25, 26, 27, 28, 28, 29, 30, 31, 33, 34, 35						
17	NP+w	MT20	2.0	4.0		
20	NP+w	MT20	2.0	4.0	1.50	1.00
31	NP+w	MT20	2.0	4.0	1.50	1.00
32	NP+t	MT20	3.0	5.0	2.00	2.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1865.2 lbs FACTORED DOWN AT 20-1-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

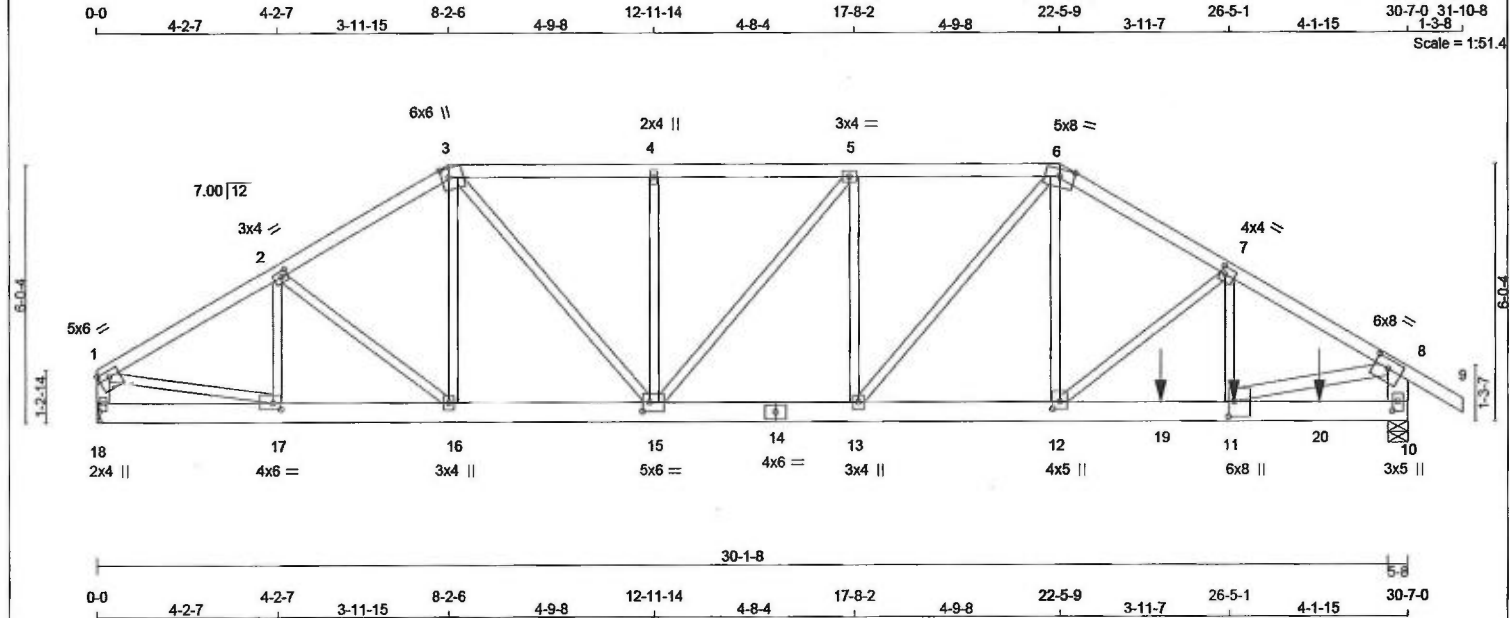
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (13) (INPUT = 0.90)
JSI METAL= 0.66 (9) (INPUT = 1.00)

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TOTAL WEIGHT = 151 lb

LUMBER

N. L. G. A. RULES				
CHORDS	SIZE		LUMBER	DESCR
1 - 3	2x4	DRY	No.2	SPF
3 - 6	2x4	DRY	No.2	SPF
6 - 9	2x4	DRY	No.2	SPF
10 - 1	2x4	DRY	No.2	SPF
10 - 8	2x6	DRY	No.2	SPF
18 - 14	2x6	DRY	No.2	SPF
14 - 10	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
11 - 8	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

IT	TYPE	PLATES	W	LEN	Y	X
1	TMWV-l	MT20	5.0	6.0	1.75	Edge
2	TMWV-l	MT20	3.0	4.0	1.50	1.75
3	TTWV+m	MT20	6.0	6.0	2.50	2.00
4	TMW-w	MT20	2.0	4.0		
5	TMWV-l	MT20	3.0	4.0		
6	TTWV-m	MT20	5.0	8.0	2.00	4.00
7	TMWV-l	MT20	4.0	4.0	2.00	1.25
8	TMWV-l	MT20	6.0	8.0	2.50	4.00
9	BMV1+p	MT20	3.0	5.0	2.75	1.50
10	BMWV+l	MT20	6.0	8.0	4.25	1.50
11	BMWV+l	MT20	4.0	5.0	2.00	2.00
12	BMWV+l	MT20	3.0	4.0		
13	BS-l	MT20	4.0	6.0		
14	BMWVW-l	MT20	5.0	6.0	2.50	2.00
15	BMWV+l	MT20	3.0	4.0		
16	BMWV-l	MT20	4.0	6.0	1.75	2.25
17	BMV1+p	MT20	2.0	4.0	2.25	1.00

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

HANGERS NOTES

1)



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ENGINEERING NOTE PAGE ENP-1. THIS
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DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

BEARINGS							
	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
18	1865	0	1865	0	0	MECHANICAL	
10	3573	0	3573	0	0	5-8	5-8

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT 18 TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
18	1309	916 / 0	0 / 0	0 / 0	0 / 0	393 / 0	0 / 0
10	2505	1766 / 0	0 / 0	0 / 0	0 / 0	739 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 10

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.78 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MAX. FACTORED		FACTORED		MAX. FACTORED				
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (C)	MAX. UNBRAC (L)	MEMB.	FORCE (LBS)	MAX CSI (C)	
FR-TO		FROM TO		LENGHTH	FR-TO			
1-2	-2470 / 0	-77.4	-77.4	0.33 (1)	4.05	17-2	-402 / 0	0.09 (1)
2-3	-2517 / 0	-77.4	-77.4	0.33 (1)	4.01	2-16	0 / 10	0.00 (4)
3-4	-2901 / 0	-77.4	-77.4	0.40 (1)	3.70	16-3	0 / 111	0.03 (4)
4-5	-2901 / 0	-77.4	-77.4	0.42 (1)	3.67	3-15	0 / 1147	0.28 (1)
5-6	-3228 / 0	-77.4	-77.4	0.46 (1)	3.47	15-4	-391 / 0	0.21 (1)
6-7	-3775 / 0	-77.4	-77.4	0.41 (1)	3.28	15-5	-510 / 0	0.52 (1)
7-8	-4764 / 0	-77.4	-77.4	0.58 (1)	2.78	13-5	-3 / 74	0.03 (4)
8-9	0 / 27	-77.4	-77.4	0.11 (1)	10.00	13-6	-76 / 0	0.08 (1)
18-1	-1812 / 0	0.0	0.0	0.20 (1)	6.16	12-6	0 / 1677	0.42 (1)
10-8	-3528 / 0	0.0	0.0	0.25 (1)	5.63	12-7	-1138 / 0	0.52 (1)
						11-7	0 / 857	0.21 (1)
18-17	0 / 0	-17.5	-17.5	0.06 (1)	10.00	1-17	0 / 2191	0.54 (1)
17-16	0 / 2148	-17.5	-17.5	0.33 (1)	10.00	11-8	0 / 4219	0.75 (1)
16-15	0 / 2158	-17.5	-17.5	0.32 (1)	10.00			
15-14	0 / 3226	-17.5	-17.5	0.50 (1)	10.00			
14-13	0 / 3226	-17.5	-17.5	0.50 (1)	10.00			
13-12	0 / 3275	-17.5	-17.5	0.36 (1)	10.00			
12-19	0 / 4126	-17.5	-17.5	0.78 (1)	10.00			
19-11	0 / 4126	-17.5	-17.5	0.78 (1)	10.00			
11-20	0 / 0	-17.5	-17.5	0.19 (1)	10.00			
20-10	0 / 0	-17.5	-17.5	0.19 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
11	26-6-4	-285	-285	---	FRONT	VERT	TOTAL
19	24-9-12	-1904	-1904	---	FRONT	VERT	TOTAL
20	28-6-4	-242	-242	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED
BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADD'TL USER-DEFINED LOADS APPLIED TO
ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.02")
CALCULATED VERT. DEFL.(LL) = L/999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (1.02")
CALCULATED VERT. DEFL.(TL) = L/999 (0.26")

CSI: TC=0.58/1.00 (7-8:1), BC=0.78/1.00 (11-12:1)
WB=0.75/1.00 (8-11:1), SS=0.78/1.00 (11-12:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP(DRY)		SHEAR		SECTION
	(PSI)		(PLI)		(PLI)
	MAX	MIN	MAX	MIN	MAX MIN
MT20	618	354	1667	822	2284 1656

CONTINUED ON PAGE 2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
NE0818-081	G02	1	1	GREENPARK - MINNISALE HOMES MILLWOOD 12 ELE 2	PAGE 6 OF 30

Version 8.210 8 Mar 12 2018 MiTek Industries, Inc. Wed Aug 15 16:46:21 2018 Page 2
ID:lbxBcPTYUOyd9wXNUTvYurz28ye-GgNpeki3HHV4FogpGIXGQFXl0gUZSXTZbNqDVNynZ3W

HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1903.7 lbs FACTORED DOWN AT 24-9-12, AND 285.0 lbs FACTORED DOWN AT 26-6-4, AND 242.2 lbs FACTORED DOWN AT 28-6-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

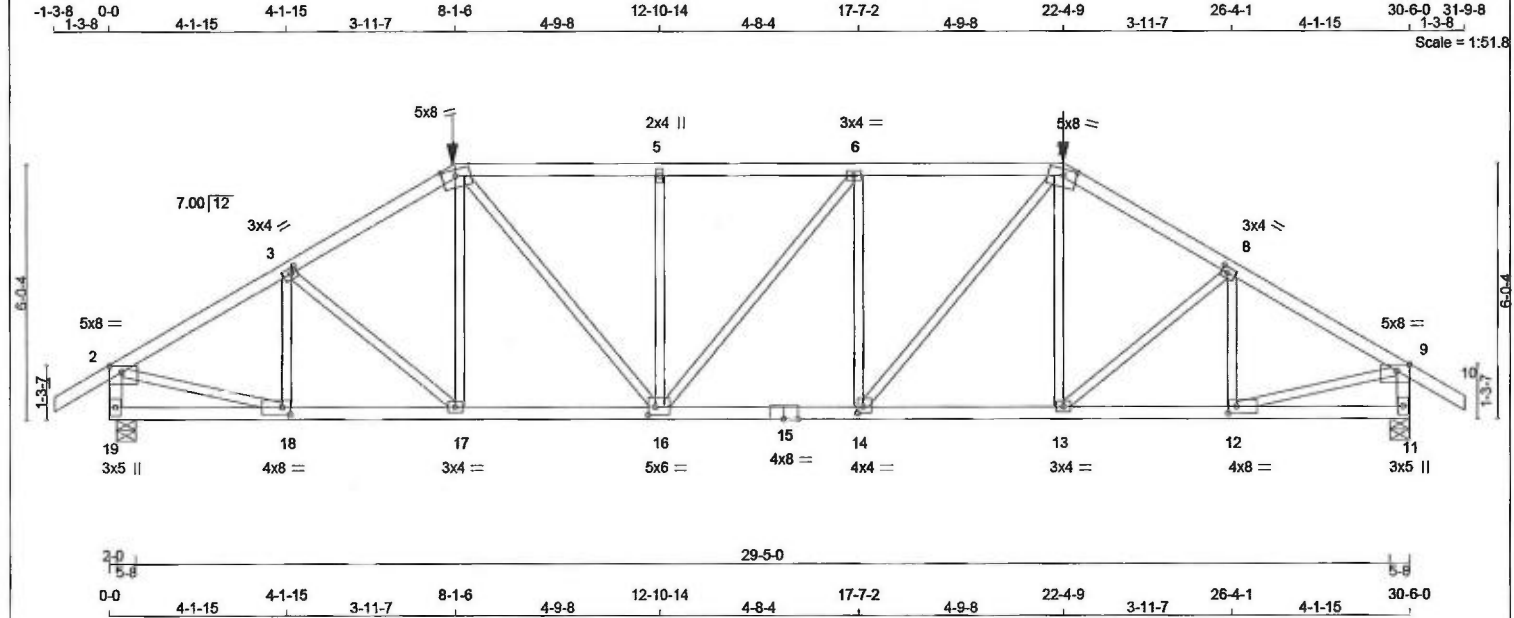
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (17) (INPUT = 0.90)
JSI METAL= 0.92 (8) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





		TOTAL WEIGHT = 133 lb	
LUMBER	DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY		(M)

N. L. G. A. RULES				BUILDING DESIGNER							DESIGN CRITERIA	
CHORDS	SIZE	LUMBER	DESCR.	BEARINGS								
				FACTORED		MAXIMUM FACTORED		INPUT		REQRD		
				GROSS REACTION		GROSS REACTION		BRG		BRG		
				JT	HPRT	DOWN	HPRT	UPLIFT	IN-SX	H-SX		
1 - 4	2x4	DRY	No.2	SPF								
4 - 7	2x4	DRY	No.2	SPF								
7 - 10	2x4	DRY	No.2	SPF								
10 - 12	2x4	DRY	No.2	SPF								
11 - 9	2x4	DRY	No.2	SPF								
19 - 15	2x4	DRY	No.2	SPF								
15 - 11	2x4	DRY	No.2	SPF								
				11	2748	0	2748	0	0	5-8	5-8	
				19	2748	0	2748	0	0	5-8	5-8	
				JT	2748	0	2748	0	0	5-8	5-8	

SPECIFIED LOADS:	
TOP CH. LL = 23.3 PSF	
DL = 3.0 PSF	
BOT CH. LL = 0.0 PSF	
DL = 7.0 PSF	
TOTAL LOAD = 33.3 PSF	

ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	UNFACTORED REACTIONS								SPACING = 24.0 IN./C/C	
					1ST LCASE MAX./MIN. COMPONENT REACTIONS									
					JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
19	1928	1352	0	0	0	0	577	0	0	0				
LOADING IN FLAT SECTION BASED ON A														

DRY; SEASONED LUMBER.	11	1928	1351/0	0/0	0/0	0/0	577/0	0/0	SLOPE OF 2.00/12 MINIMUM
BEARING MATERIAL TO BE SPEC NO 2 OR BETTER AT JOINT(S) 19, 11									GIRDER TYPE: C-Primekin

BEARING MATERIAL TO BE SFT NO.2 OR BETTER AT JOINT(S) IS, IT

PLATES (table is in inches)						BRACING		RIGHT SETBACK = 8-17	
JT TYPE	PLATES	W	LEN	Y	X	TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.69 FT.		END SETBACK = 6-00	
2 TMVW-p	MT20	5.0	8.0	1.75	3.50	MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.		END WALL WIDTH = 5-8	
								CORNER FRAMING TYPE: CONVENTIONAL	

3	TMWW-t	MT20	3.0	4.0	1.50	1.75	END JACK TYPE: CONVENTIONAL
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4	TTWW-m	MT20	5.0	8.0	1.75	3.75	ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.	APPLIED TO BACK SIDE
5	TMW+w	MT20	2.0	4.0				- ADD'L LOADS BASED ON 55 % OF GSL.
6	TMWW-t	MT20	3.0	4.0			LOADING	
7	TTWW-m	MT20	5.0	8.0	1.75	3.75	TOTAL LOAD CASES: (4)	THIS TRUSS IS DESIGNED FOR RESIDENTIAL

[illegible]

9	TMVW-p	M120	5.0	8.0	1.75	3.50	CHORDS				WEBS				PART 9, NBCC 2010			
10	BMV1+p	M120	3.0	5.0			MAX. FACTORED		FACTORED				MAX. FACTORED					
12	BMWW-I	M120	4.0	8.0	2.00	2.25	MEMB.	FORCE	VERT. LOAD	LC1	MAX	MAX.	MEMB.	FORCE	MAX			
13	BMWW-I	M120	3.0	4.0				(LBS)	(PLF)		CSI (LC)	UNBRAC		(LBS)	CSI (LC)			

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , 8CBC 2012 , ABC 2014

FR-TO	FROM TO	LENGTH FR-TO
14 BMW-t MT20 4.0 4.0 1.75 1.50	77.4 77.4 0.11 (1)	40.00 18.3 -568 / 0
15 BSA MT20 4.0 4.0 1.75 1.50	77.4 77.4 0.11 (1)	40.00 18.3 -568 / 0

15	BSR	MT20	4.0	6.0		2	-2	-3454 / 0	-77.4	-77.4	0.1	3.42	3-17	0/168	0.04 (t)	(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED RAFTER LOAD.
16	BMWWWW-I	MT20	5.0	6.0	2.25	2.00	2-3	-3454 / 0	-77.4	-77.4	0.41 (t)	3.42	3-17	0/168	0.04 (t)	
17	BMWW-I	MT20	3.0	4.0			4-3	-3639 / 0	-77.4	-77.4	0.42 (t)	3.31	17-4	0/160	0.06 (t)	
18	BMWW-I	MT20	4.0	8.0	2.00	2.25	4-5	-3878 / 0	-145.9	-145.9	0.79 (t)	2.72	4-16	0/1170	0.29 (t)	

19	BMV1+D	M120	3.0	3.0	3-6	-3878 / 0	-143.9	-143.9	0.79 (1)	2.70	16-3	-744 / 0	0.43 (1)	ROOF LIVE LOAD
					6-7	-3881 / 0	-145.9	-145.9	0.80 (1)	2.69	16-6	-5 / 0	0.01 (1)	

7-8	-3640 / 0	-77.4	-77.4	0.42 (1)	3.31	14-6	-745 / 0	0.43 (1)	ALLOWABLE DEFL (LL) = $\frac{1}{360}$ (1.02")
8-9	-3457 / 0	-77.4	-77.4	0.41 (1)	3.42	14-7	0 / 1174	0.29 (1)	CALCULATED VERT. DEFL. (LL) = $\frac{1}{999}$ (0.18")
9-10	0 / 27	-77.4	-77.4	0.11 (1)	10.00	13-7	0 / 160	0.06 (4)	ALLOWABLE DEFL (TL) = $\frac{1}{360}$ (1.02")
1)	-2684 / 0	0.0	0.0	0.30 (1)	5.39	13-8	0 / 166	0.04 (1)	CALCULATED VERT. DEFL. (TL) = $\frac{1}{999}$ (0.32")

REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 471.1 lbs FACTORED DOWN AT 22-4-9,	11-2	2004 / 0	0.0	0.0	0.00 (1)	5.19	12-8	-566 / 0	0.13 (1)	DESIGNED PER: SEE (12-8-2004) (12-8-2004)
	11-9	-2684 / 0	0.0	0.0	0.30 (1)	5.19	12-8	-566 / 0	0.13 (1)	

AND 470.8 lbs FACTORED DOWN AT 8-1-6 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.	19-18	0 / 0	-33.0	-33.0	0.13 (4)	10.00	2-18	0 / 3078	0.76 (1)	CS: TC=0.80/1.00 (6-7:1), BC=0.78/1.00 (14-16:1)
	18-17	0 / 2997	-33.0	-33.0	0.61 (1)	10.00	12-9	0 / 3079	0.76 (1)	, WB=0.76/1.00 (9-12:1), SSI=0.36/1.00 (6-7:1)
	17-16	0 / 3127	-33.0	-33.0	0.64 (1)	10.00				DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00

16-15	0 / 3881	-33.0	-33.0	0.76 (1)	10.00	COMP=1.00 SHEAR=1.00 TENS= 1.00
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15-14	0 / 3881	-33.0	-33.0	0.76 (1)	10.00	COMPANION LIVE LOAD FACTOR = 0.50
14-13	0 / 3127	-33.0	-33.0	0.63 (1)	10.00	
13-12	0 / 2999	-33.0	-33.0	0.61 (1)	10.00	
12-11	0 / 0	-33.0	-33.0	0.13 (4)	10.00	

AUTOSOLVE RIGHT HEEL ONLY

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
4	8-1-6	-471	-471	—	BACK	VERT	TOTAL
7	22-4-9	-471	-471	—	BACK	VERT	TOTAL

TEST NO.	TEST DATE	TESTER	TEST TYPE	TEST RESULT	NAIL VALUES	
					PLATE GRIP (DRY) SHEAR	SECTION
1	10/10/10	J. K. K. K.	1	1000	1000	1000
2	10/10/10	J. K. K. K.	1	1000	1000	1000
3	10/10/10	J. K. K. K.	1	1000	1000	1000
4	10/10/10	J. K. K. K.	1	1000	1000	1000
5	10/10/10	J. K. K. K.	1	1000	1000	1000
6	10/10/10	J. K. K. K.	1	1000	1000	1000
7	10/10/10	J. K. K. K.	1	1000	1000	1000
8	10/10/10	J. K. K. K.	1	1000	1000	1000
9	10/10/10	J. K. K. K.	1	1000	1000	1000
10	10/10/10	J. K. K. K.	1	1000	1000	1000


 Aug 15, 2018

	PSI	PLI	PLI	PLI
	MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656	

CONTINUED ON PAGE 2

READ ALL NOTES ON THIS PAGE AND ON		
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ENGINEERING NOTE PAGE ENP-1. THIS

NOTE PAGE IS AN INTEGRAL PART OF

THIS DRAWING AS IT CONTAINS

THIS DRAWING AND IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN

SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT

THE DESIGN OF THIS COMPONENT.

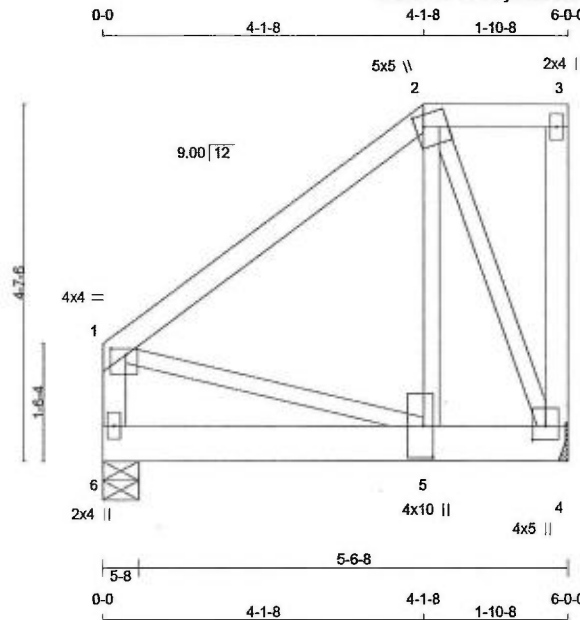
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
NE0818-081	G03	1	1	GREENPARK - MINNISALE HOMES MILLWOOD 12 ELE 2	PAGE 8 OF 30
TRUSS DESC.					

Version 8.210 S Mar 12 2018 MiTek Industries, Inc. Wed Aug 15 16:46:22 2018 Page 2
ID:lbxBcPTYUOyd9wXNUTvYurz28ye-ktxBR4jh2bextyF0qS2VyS4QM4r8B Sj1am2pynZ3V

PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.
JSI GRIP= 0.89 (7) (INPUT = 0.90)
JSI METAL= 0.77 (15) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.





Scale = 1:28.5

TOTAL WEIGHT = 34 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	DRY	LUMBER	DESCR.
1 - 2	2x4	DRY	No.2	SPF
2 - 3	2x4	DRY	No.2	SPF
4 - 3	2x4	DRY	No.2	SPF
6 - 1	2x4	DRY	No.2	SPF
6 - 4	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
DRY: SEASONED LUMBER.				

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
1	TMVW-p	MT20	4.0	4.0	1.00	2.25
2	TTWW+m	MT20	5.0	5.0	1.75	1.25
3	TMV-p	MT20	2.0	4.0		
4	BMVW1+p	MT20	4.0	5.0	2.25	2.00
5	BMWW+t	MT20	4.0	10.0	5.00	1.50
6	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX
4	1903	0	1903	0	0	MECHANICAL
6	1903	0	1903	0	0	5-8

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT 4 TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
4	1336	934 / 0	0 / 0	0 / 0	0 / 0	401 / 0	0 / 0
6	1336	934 / 0	0 / 0	0 / 0	0 / 0	401 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 6

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.12 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		WEBS		FACTORED	
	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (PLF)	LC1 MAX (LC)	MAX. UNBRACED LENGTH	MEMB. FORCE (LBS)	MAX. FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM	TO		FR-TO			
1-2	-894 / 0	-77.4	-77.4	0.27 (1)	6.12	5-2	0 / 2027	0.50 (1)
2-3	0 / 0	-77.4	-77.4	0.05 (1)	10.00	2-4	-1882 / 0	0.63 (1)
4-3	-73 / 0	0.0	0.0	0.02 (1)	7.81	1-5	0 / 738	0.18 (1)
6-1	-885 / 0	0.0	0.0	0.10 (1)	7.81			
6-5	0 / 0	-557.0	-557.0	0.68 (1)	10.00			
5-4	0 / 764	-557.0	-557.0	0.60 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
	DL = 3.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CSldGirder

START DISTANCE = 0-0

START SPAN CARRIED = 24-9-0

END DISTANCE = 6-0-0

END SPAN CARRIED = 24-9-0

END WALL WIDTH = 0-0

APPLIED TO BACK SIDE OF BOTTOM CHORD.

- ADD'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/730 (0.10")

CSI: TC=0.27/1.00 (1-2:1), BC=0.68/1.00 (5-6:1), WB=0.63/1.00 (2-4:1), SI=0.89/1.00 (5-6:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

CONTINUED ON PAGE 2



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
NE0818-081	G04	1	1	GREENPARK - MINNISALE HOMES MILLWOOD 12 ELE 2	PAGE 10 OF 30

Version 8.210 S Mar 12 2018 MiTek Industries, Inc. Wed Aug 15 16:46:22 2018 Page 2
ID:lbxBcPTYUOyd9wXNUTvYurz28ye-ktxBr4jh2bexyF0qS2VyS4Yd4slB0Tjq1am2pynZ3V

PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.
JSI GRIP= 0.89 (2) (INPUT = 0.90)
JSI METAL= 0.52 (2) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON
ENGINEERING NOTE PAGE ENP-1. THIS
NOTE PAGE IS AN INTEGRAL PART OF
THIS DRAWING AS IT CONTAINS
SPECIFICATIONS AND CRITERIA USED IN
THE DESIGN OF THIS COMPONENT.

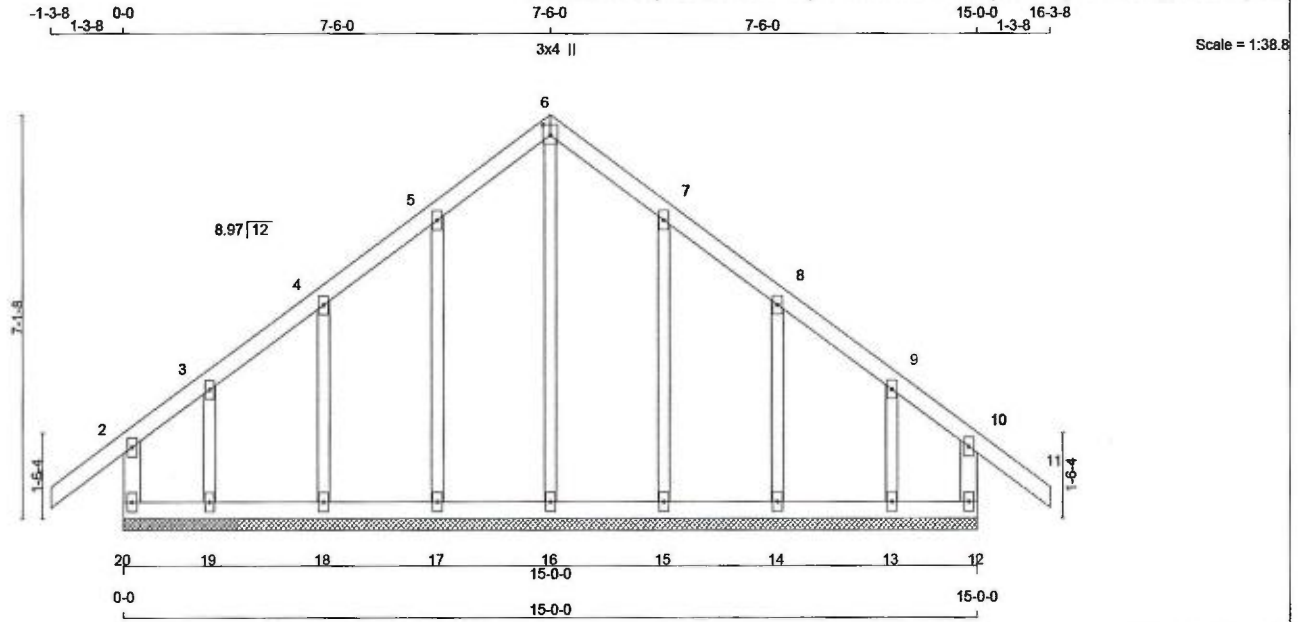


ID:lbxBcPTYUOyd9wXNUTvYurz28ye-C3VZ3QkJoumoU6qCNAakVgdj4TLlwcpS2hJKaGynZ3U

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION
 (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 822 2284 1656
 PLATE PLACEMENT TOL. = 0.250 inches
 PLATE ROTATION TOL. = 5.0 Deg.
 JSI GRIP= 0.34 (2) (INPUT = 0.90)
 JSI METAL= 0.08 (2) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON
 ENGINEERING NOTE PAGE ENP-1. THIS
 NOTE PAGE IS AN INTEGRAL PART OF
 THIS DRAWING AS IT CONTAINS
 SPECIFICATIONS AND CRITERIA USED IN
 THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 68 lb [M]

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
20 - 2	2x4	DRY	No.2	SPF
1 - 6	2x4	DRY	No.2	SPF
6 - 11	2x4	DRY	No.2	SPF
12 - 10	2x4	DRY	No.2	SPF
20 - 12	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

GABLE STUDS SPACED AT 2'-0" O.C.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMV+p	MT20	2.0	4.0		
3, 4, 5, 7, 8, 9						
3	TMW+w	MT20	2.0	4.0		
6	TTW+p	MT20	3.0	4.0	2.25	1.50
10	TMV+p	MT20	2.0	4.0		
12	BMV1+p	MT20	2.0	4.0		
13, 14, 15, 16, 17, 18, 19						
13	BMW1+w	MT20	2.0	4.0		
20	BMV1+p	MT20	2.0	4.0		

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	UNBRAC LENGTH
FR-TO		FROM TO		FR-TO			
20-2	-177 / 0	0.0	0.0 0.03 (1)	7.81	16-6	-195 / 0	0.17 (1)
1-2	0 / 32	-77.4	-77.4 0.11 (1)	10.00	17-5	-157 / 0	0.08 (1)
2-3	0 / 2	-77.4	-77.4 0.07 (1)	10.00	18-4	-160 / 0	0.04 (1)
3-4	0 / 27	-77.4	-77.4 0.04 (1)	10.00	19-3	-96 / 0	0.02 (1)
4-5	0 / 27	-77.4	-77.4 0.04 (1)	10.00	15-7	-157 / 0	0.08 (1)
5-6	0 / 29	-77.4	-77.4 0.04 (1)	10.00	14-8	-160 / 0	0.04 (1)
6-7	0 / 29	-77.4	-77.4 0.04 (1)	10.00	13-9	-96 / 0	0.02 (1)
7-8	0 / 27	-77.4	-77.4 0.04 (1)	10.00			
8-9	0 / 27	-77.4	-77.4 0.04 (1)	10.00			
9-10	0 / 2	-77.4	-77.4 0.07 (1)	10.00			
10-11	0 / 32	-77.4	-77.4 0.11 (1)	10.00			
12-10	-177 / 0	0.0	0.0 0.03 (1)	7.81			
20-19	-14 / 0	-17.5	-17.5 0.01 (4)	6.25			
19-18	-18 / 0	-17.5	-17.5 0.01 (4)	6.25			
18-17	-22 / 0	-17.5	-17.5 0.01 (4)	6.25			
17-16	-24 / 0	-17.5	-17.5 0.01 (4)	6.25			
16-15	-24 / 0	-17.5	-17.5 0.01 (4)	6.25			
15-14	-22 / 0	-17.5	-17.5 0.01 (4)	6.25			
14-13	-18 / 0	-17.5	-17.5 0.01 (4)	6.25			
13-12	-14 / 0	-17.5	-17.5 0.01 (4)	6.25			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11/1.00 (1-2:1), BC=0.01/1.00 (17-18:4), WB=0.17/1.00 (6-16:1), SSI=0.07/1.00 (1-2:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

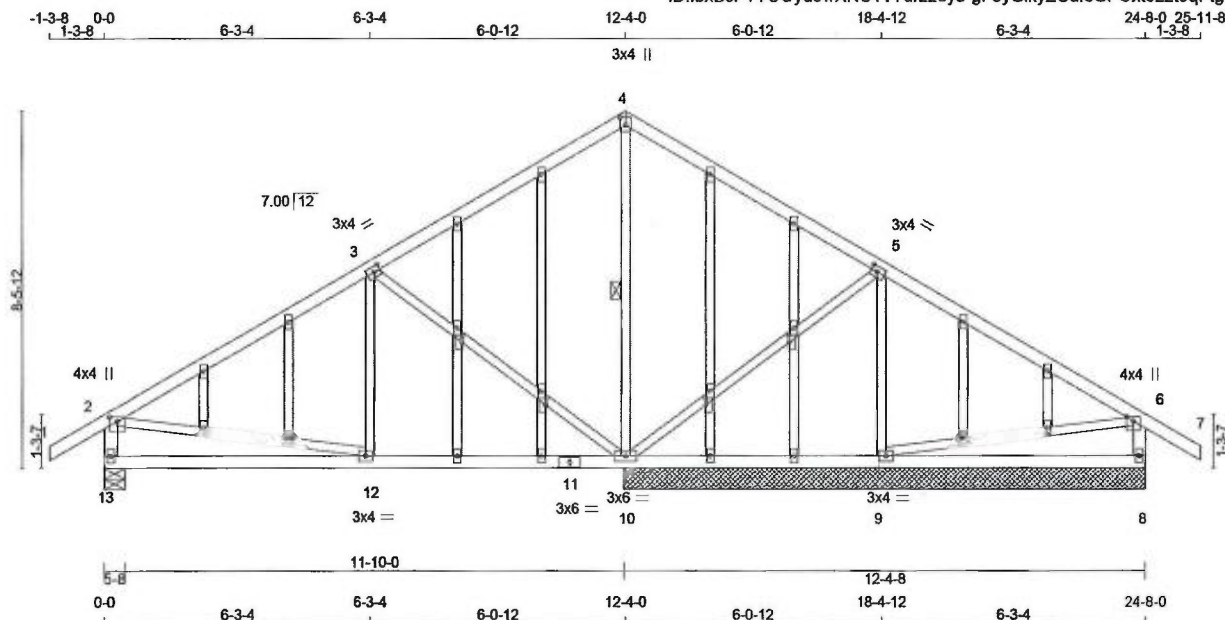
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (6) (INPUT = 0.90)
JSI METAL= 0.07 (6) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 128 lb [M]

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF	
4 - 7	2x4	DRY	No.2	SPF	
13 - 2	2x4	DRY	No.2	SPF	
8 - 6	2x4	DRY	No.2	SPF	
13 - 11	2x4	DRY	No.2	SPF	
11 - 8	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF EXCEPT

ALL GABLE WEBS 2x3 DRY No.2 SPF DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2'-0" O.C.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW+p	MT20	4.0	4.0	1.25	2.00
3	TMVW-t	MT20	3.0	4.0	1.50	1.75
4	TTW+p	MT20	3.0	4.0	2.25	1.50
5	TMVW-t	MT20	3.0	4.0	1.50	1.75
6	TMVW+p	MT20	4.0	4.0	1.25	2.00
8	BMV1+p	MT20	2.0	4.0		
9	BMVW1-t	MT20	3.0	4.0		
10	BMVW1-t	MT20	3.0	6.0		
11	BS-t	MT20	3.0	6.0		
12	BMVW-t	MT20	3.0	4.0		
13	BMV1+p	MT20	2.0	4.0		
14, 17, 26, 29						
14	NP+w	MT20	2.0	4.0	2.00	0.25
14, 15, 16, 17, 18, 19, 20, 21, 22, 23, 24, 25, 26, 27, 28, 29, 30, 31, 32, 33						
14	NP+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
13	607	0	607	0	0	5-8	5-8		
10	1079	0	1079	0	0	12-4-8	12-4-8		
9	507	0	507	0	0	12-4-8	12-4-8		
8	358	0	358	0	0	12-4-8	12-4-8		

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
13	423	312 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0
10	757	530 / 0	0 / 0	0 / 0	0 / 0	227 / 0	0 / 0
9	358	239 / 0	0 / 0	0 / 0	0 / 0	119 / 0	0 / 0
8	247	195 / 0	0 / 0	0 / 0	0 / 0	52 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 13, 10, 9, 8

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF 4-10. DBS = 20-0-0. CBF = 61 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
1-2	0 / 27	-77.4 -77.4	0.10 (1)	10.00	12-3	0 / 122	0.04 (4)
2-3	-398 / 0	-77.4 -77.4	0.52 (1)	6.25	3-10	-615 / 0	0.72 (1)
3-4	0 / 109	-77.4 -77.4	0.51 (1)	10.00	10-4	-488 / 0	0.22 (1)
4-5	0 / 108	-77.4 -77.4	0.52 (1)	10.00	10-5	-193 / 0	0.23 (1)
5-6	-9 / 14	-77.4 -77.4	0.50 (1)	10.00	9-5	-390 / 0	0.14 (1)
6-7	0 / 27	-77.4 -77.4	0.10 (1)	10.00	2-12	0 / 373	0.08 (1)
13-2	-562 / 0	0.0 0.0	0.06 (1)	7.81	9-6	-9 / 34	0.01 (1)
8-6	-314 / 0	0.0 0.0	0.03 (1)	7.81			
13-12	0 / 0	-17.5 -17.5	0.16 (4)	10.00			
12-11	0 / 369	-17.5 -17.5	0.19 (4)	10.00			
11-10	0 / 369	-17.5 -17.5	0.19 (4)	10.00			
10-9	-9 / 33	-17.5 -17.5	0.17 (4)	10.00			
9-8	0 / 0	-17.5 -17.5	0.17 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL)= L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL)= L/999 (0.05")

CSI: TC=0.52/1.00 (2-3:1), BC=0.19/1.00 (10-12:4)
WB=0.72/1.00 (3-10:1), SSI=0.20/1.00 (5-6:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

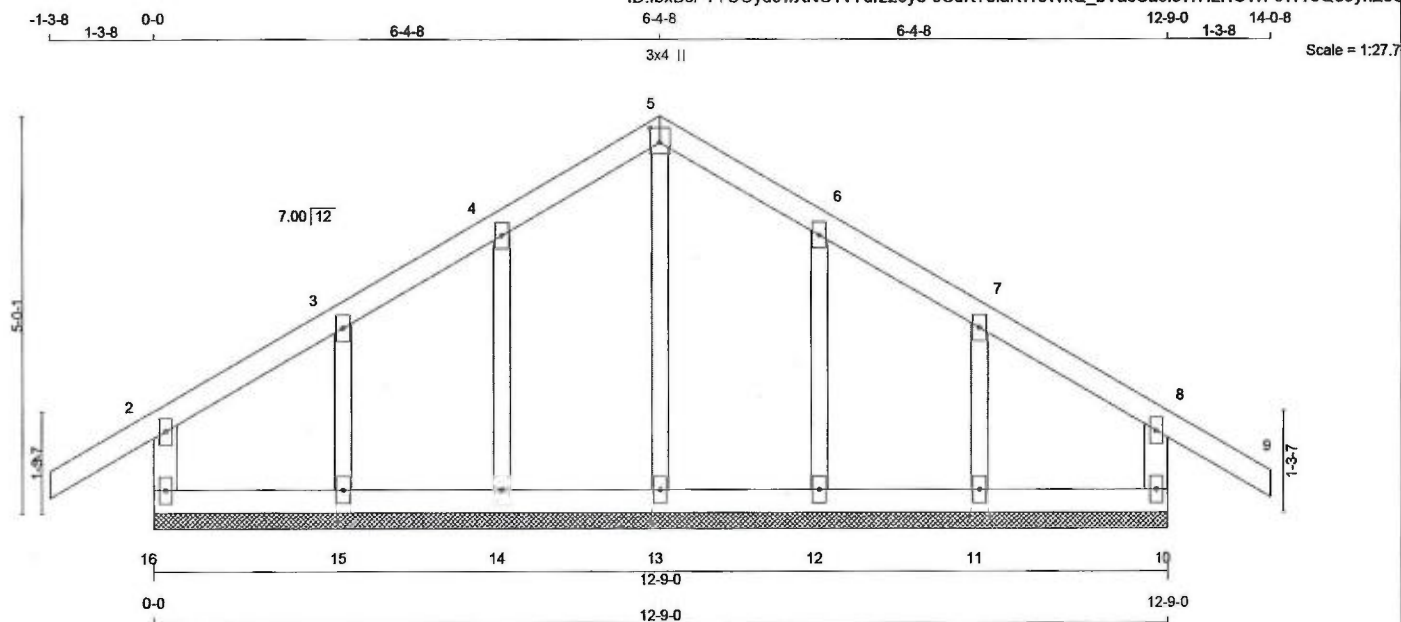
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (12) (INPUT = 0.90)
JSI METAL= 0.14 (12) (INPUT = 1.00)



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TOTAL WEIGHT = 49 lb [M]

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
16 - 2	2x4	DRY	No.2
1 - 5	2x4	DRY	No.2
5 - 9	2x4	DRY	No.2
10 - 8	2x4	DRY	No.2
16 - 10	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			
GABLE STUDS SPACED AT	2-0-0	OC.	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	UNBRAC LENGTH
16-2	-209 / 0	0.0	0.0 (1)	7.81	13-5	-150 / 0	0.05 (1)
1-2	0 / 27	-77.4	-77.4 (1)	10.00	14-4	-153 / 0	0.04 (1)
2-3	-24 / 0	-77.4	-77.4 (1)	6.25	15-3	-161 / 0	0.03 (1)
3-4	-17 / 0	-77.4	-77.4 (1)	6.25	12-6	-153 / 0	0.04 (1)
4-5	-12 / 0	-77.4	-77.4 (1)	6.25	11-7	-161 / 0	0.03 (1)
5-6	-12 / 0	-77.4	-77.4 (1)	6.25			
6-7	-17 / 0	-77.4	-77.4 (1)	6.25			
7-8	-24 / 0	-77.4	-77.4 (1)	6.25			
8-9	0 / 27	-77.4	-77.4 (1)	10.00			
10-8	-209 / 0	0.0	0.0 (1)	7.81			
16-15	0 / 19	-17.5	-17.5 (4)	10.00			
15-14	0 / 14	-17.5	-17.5 (4)	10.00			
14-13	0 / 10	-17.5	-17.5 (4)	10.00			
13-12	0 / 10	-17.5	-17.5 (4)	10.00			
12-11	0 / 14	-17.5	-17.5 (4)	10.00			
11-10	0 / 19	-17.5	-17.5 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.10/1.00 (1-2:1), BC=0.02/1.00 (10-11:4), WB=0.05/1.00 (5-13:1), SSI=0.07/1.00 (1-2:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

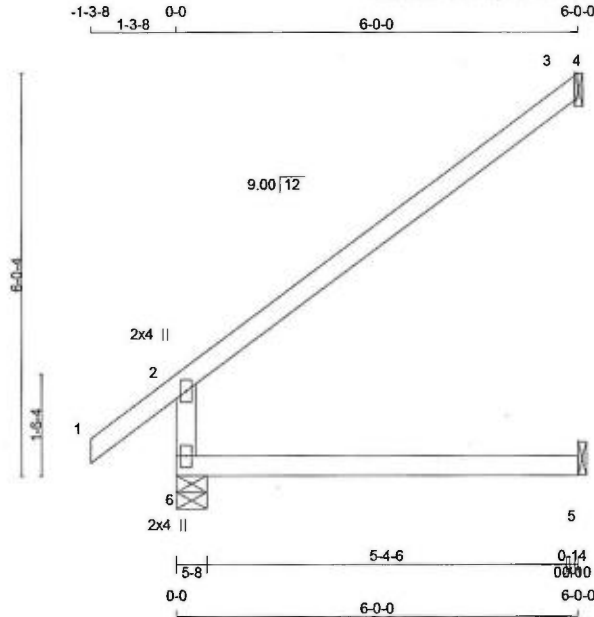
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (8) (INPUT = 0.90)
JSI METAL = 0.06 (5) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:33.0

TOTAL WEIGHT = 15 X 19 = 281 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.	SPF
6 - 2	2x4	DRY	No.2	SPF	
1 - 4	2x4	DRY	No.2	SPF	
6 - 5	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMV+p	MT20	2.0	4.0		
6	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
6	452	0	452	0	5-8	5-8
5	44	0	50	0	1-8	1-8
3	172	0	172	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) 5, 3

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
6	314	236 / 0	0 / 0	0 / 0	0 / 0	79 / 0	0 / 0
5	35	0 / 0	0 / 0	0 / 0	0 / 0	35 / 0	0 / 0
3	117	103 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 6

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED			MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRAC LENGTH	FACTORED CSI (LC)
		VERT. LOAD (PLF)	LC1	MAX. CSI (LC)				
FR-TO		FROM	TO		FR-TO			
6-2	-391 / 0	0.0	0.0	0.12 (4)	7.81			
1-2	0 / 32	-77.4	-77.4	0.11 (1)	10.00			
2-3	-34 / 0	-77.4	-77.4	0.46 (1)	6.25			
3-4	0 / 0	-7.5	-7.5	0.00 (4)	10.00			
6-5	0 / 0	-17.5	-17.5	0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.46/1.00 (2-3:1), BC=0.13/1.00 (5-6:4), WB=0.00/1.00 (n/a:0), SSI=0.18/1.00 (2-3:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

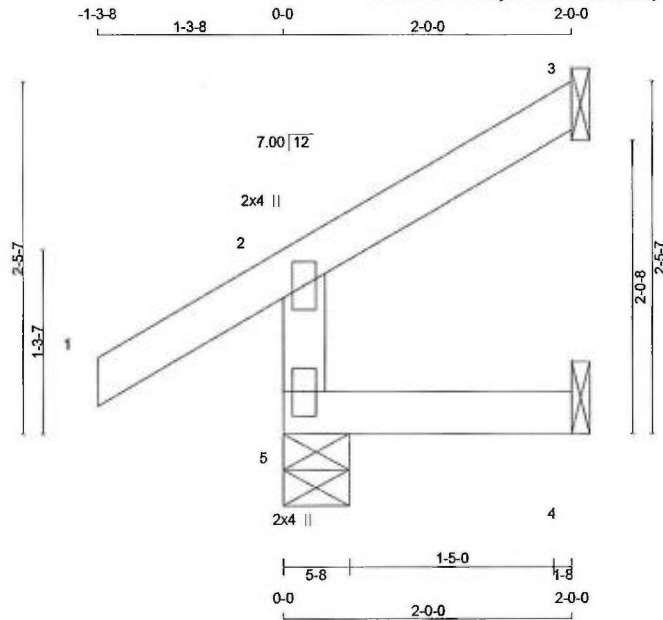
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (2) (INPUT = 0.90)
JSI METAL = 0.10 (2) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:15.3

TOTAL WEIGHT = 2 X 8 = 16 lb [M]

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	DRY
5 - 2	2x4	DRY	No.2
1 - 3	2x4	DRY	No.2
5 - 4	2x4	DRY	No.2

DESCR. LUMBER SPF

SPF No.2 No.2 No.2

SPF No.2 No.2 No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
2	TMV+p	MT20	2.0	4.0	
5	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
5	221	0	221	0	5-8	5-8
3	58	0	58	0	1-8	1-8
4	16	0	16	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) 3, 4

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
5	153	122 / 0	0 / 0	0 / 0	0 / 0	31 / 0	0 / 0
3	40	35 / 0	0 / 0	0 / 0	0 / 0	5 / 0	0 / 0
4	13	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 5

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 MAX CSI (LC)	MAX. UNBRACED LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
5-2	-202 / 0	0.0	0.0 0.01 (4)	7.81			
1-2	0 / 27	-77.4	-77.4 0.10 (1)	10.00			
2-3	-10 / 0	-77.4	-77.4 0.05 (1)	10.00			
5-4	0 / 0	-17.5	-17.5 0.02 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
	DL = 3.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.10/1.00 (1-2:1), BC=0.02/1.00 (4-5:4), WB=0.00/1.00 (n/a:0), SSI=0.07/1.00 (1-2:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1658

PLATE PLACEMENT TOL. = 0.250 inches

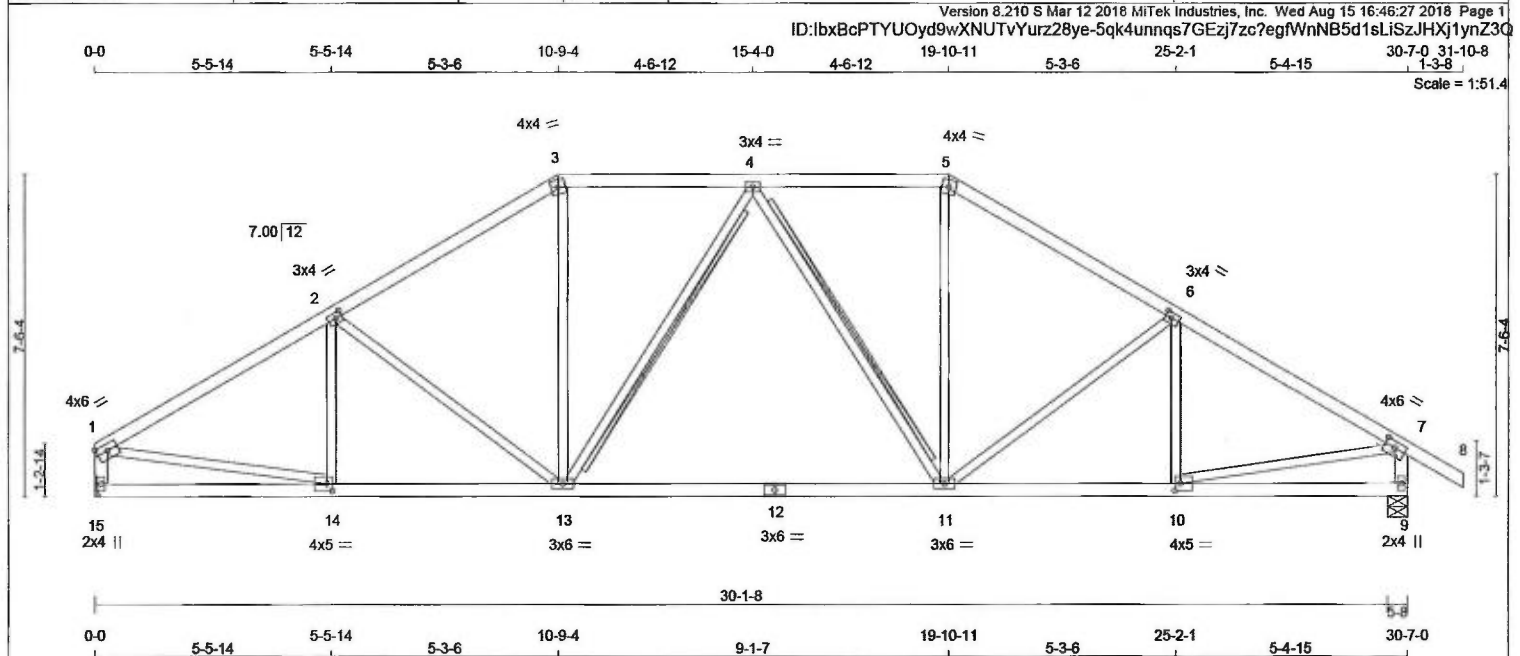
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.12 (2) (INPUT = 0.90)
JSI METAL = 0.05 (2) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
1 - 3	2x4	DRY	No.2	SPF
3 - 5	2x4	DRY	No.2	SPF
5 - 8	2x4	DRY	No.2	SPF
15 - 1	2x4	DRY	No.2	SPF
9 - 7	2x4	DRY	No.2	SPF
15 - 12	2x4	DRY	No.2	SPF
12 - 9	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
1	TMVW-t	MT20	4.0	6.0	1.75	Edge
2	TMWW-t	MT20	3.0	4.0	1.50	1.75
3	TTW-m	MT20	4.0	4.0		
4	TMWW-t	MT20	3.0	4.0		
5	TTW-m	MT20	4.0	4.0		
6	TMWW-t	MT20	3.0	4.0	1.50	1.75
7	TMVW-t	MT20	4.0	6.0	1.75	3.00
9	BMV1+p	MT20	2.0	4.0		
10	BMWW-t	MT20	4.0	5.0	2.00	1.50
11	BMWW-t	MT20	3.0	6.0		
12	BS-t	MT20	3.0	6.0		
13	BMWW-t	MT20	3.0	6.0		
14	BMWW-t	MT20	4.0	5.0	2.00	1.50
15	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
15	1451	0	1451	0	MECHANICAL	
9	1556	0	1556	0	5-8	5-8

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT 15 TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST LCASE MAX./MIN. COMPONENT REACTIONS						
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
15	1018	712 / 0	0 / 0	0 / 0	0 / 0	306 / 0	0 / 0
9	1090	776 / 0	0 / 0	0 / 0	0 / 0	314 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 9

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.57 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT 4-13, 4-11

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
1-2	-1890 / 0	-77.4 -77.4	0.35 (1)	4.57 14-2	-192 / 21	0.06 (1)	
2-3	-1661 / 0	-77.4 -77.4	0.32 (1)	4.83 2-13	-304 / 0	0.26 (1)	
3-4	-1419 / 0	-77.4 -77.4	0.22 (1)	5.25 13-3	0 / 533	0.12 (1)	
4-5	-1415 / 0	-77.4 -77.4	0.22 (1)	5.26 13-4	-247 / 0	0.14 (1)	
5-6	-1655 / 0	-77.4 -77.4	0.32 (1)	4.84 4-11	-256 / 0	0.14 (1)	
6-7	-1867 / 0	-77.4 -77.4	0.34 (1)	4.60 11-5	0 / 530	0.12 (1)	
7-8	0 / 27	-77.4 -77.4	0.10 (1)	10.00 11-6	-285 / 0	0.24 (1)	
15-1	-1407 / 0	0.0 0.0	0.14 (1)	6.87 10-6	-208 / 15	0.06 (1)	
9-7	-1513 / 0	0.0 0.0	0.15 (1)	6.68 1-14	0 / 1676	0.38 (1)	
				10-7	0 / 1660	0.37 (1)	
15-14	0 / 0	-17.5 -17.5	0.11 (4)	10.00			
14-13	0 / 1653	-17.5 -17.5	0.39 (1)	10.00			
13-12	0 / 1549	-17.5 -17.5	0.38 (1)	10.00			
12-11	0 / 1549	-17.5 -17.5	0.38 (1)	10.00			
11-10	0 / 1634	-17.5 -17.5	0.39 (1)	10.00			
10-9	0 / 0	-17.5 -17.5	0.11 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.02")
CALCULATED VERT. DEFL.(LL)= L/ 899 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (1.02")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.23")

CSI: TC=0.35/1.00 (1-2:1), BC=0.39/1.00 (13-14:1), WB=0.38/1.00 (1-14:1), SSI=0.18/1.00 (1-2:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

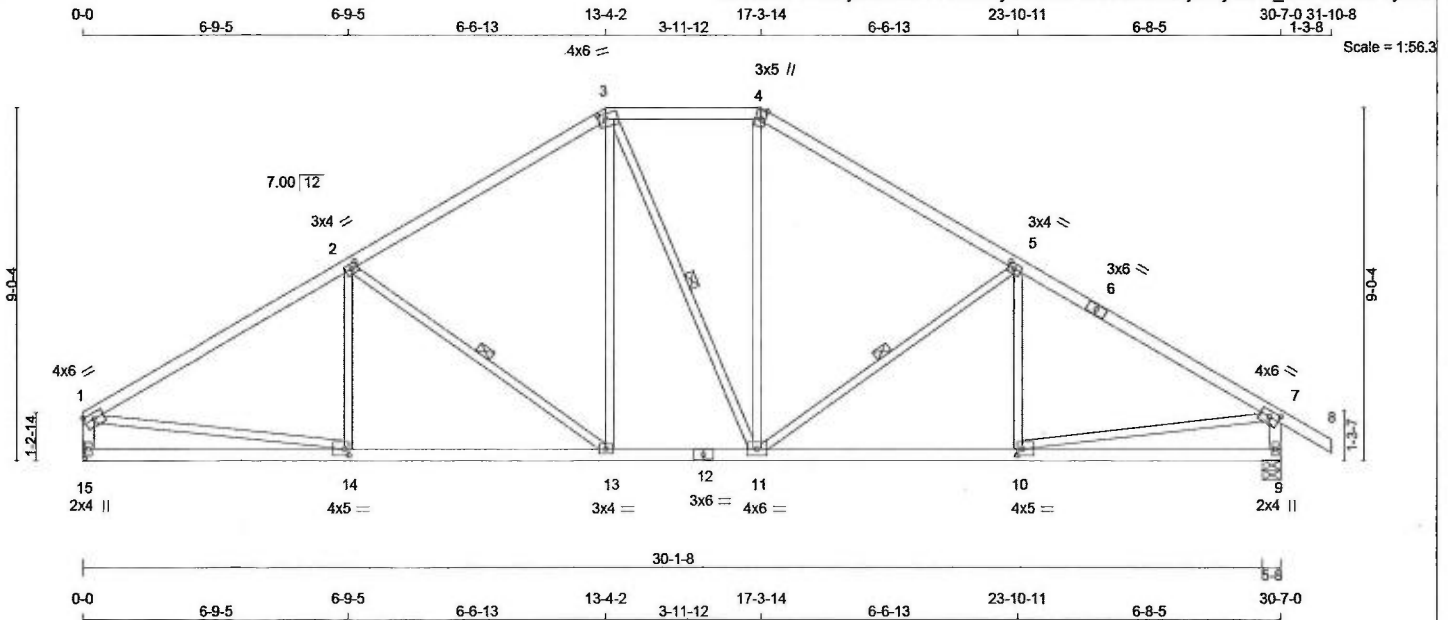
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (10) (INPUT = 0.90)
JSI METAL= 0.56 (12) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 131 lb
[M][F]

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
1 - 3	2x4	DRY	No.2
3 - 4	2x4	DRY	No.2
4 - 6	2x4	DRY	No.2
6 - 8	2x4	DRY	No.2
15 - 1	2x4	DRY	No.2
9 - 7	2x4	DRY	No.2
15 - 12	2x4	DRY	No.2
12 - 9	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2

EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
1	TMVW-t	MT20	4.0	6.0	1.75	Edge
2	TMVW-t	MT20	3.0	4.0	1.50	1.75
3	TTWW-m	MT20	4.0	6.0	1.75	2.25
4	TTW+m	MT20	3.0	5.0	2.75	1.25
5	TMVW-t	MT20	3.0	4.0	1.50	1.75
6	TS-t	MT20	3.0	6.0		
7	TMVW-t	MT20	4.0	6.0	1.75	3.00
9	BMV1+p	MT20	2.0	4.0		
10	BMVW-t	MT20	4.0	5.0	2.00	1.50
11	BMVWW-t	MT20	4.0	6.0		
12	BS-t	MT20	3.0	6.0		
13	BMVW-t	MT20	3.0	4.0		
14	BMVW-t	MT20	4.0	5.0	2.00	1.50
15	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
15	1451	0	1451	0	5-8	5-8
9	1556	0	1556	0	5-8	5-8

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT 15 TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM.LIVE	LIVE			
15	1018	712 / 0	0 / 0	0 / 0	0 / 0	306 / 0	0 / 0
9	1090	776 / 0	0 / 0	0 / 0	0 / 0	314 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 9

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.30 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF 2-13, 3-11, 5-11. DBS = 20-0-0. CBF = 65 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
1-2	-1902 / 0	-77.4 -77.4	0.56 (1)	4.30	14-2	-94 / 87	0.04 (1)
2-3	-1482 / 0	-77.4 -77.4	0.50 (1)	4.80	2-13	-518 / 0	0.24 (1)
3-4	-1255 / 0	-77.4 -77.4	0.18 (1)	5.56	13-3	0 / 389	0.09 (1)
4-5	-1481 / 0	-77.4 -77.4	0.50 (1)	4.81	3-11	-2 / 0	0.00 (1)
5-6	-1884 / 0	-77.4 -77.4	0.54 (1)	4.33	11-4	0 / 386	0.09 (1)
6-7	-1884 / 0	-77.4 -77.4	0.54 (1)	4.33	11-5	-500 / 0	0.23 (1)
7-8	0 / 27	-77.4 -77.4	0.10 (1)	10.00	10-5	-108 / 81	0.04 (1)
15-1	-1402 / 0	0.0 0.0	0.14 (1)	6.88	1-14	0 / 1685	0.38 (1)
9-7	-1508 / 0	0.0 0.0	0.15 (1)	6.68	10-7	0 / 1672	0.38 (1)
15-14	0 / 0	-17.5 -17.5	0.20 (4)	10.00			
14-13	0 / 1669	-17.5 -17.5	0.37 (1)	10.00			
13-12	0 / 1256	-17.5 -17.5	0.26 (1)	10.00			
12-11	0 / 1256	-17.5 -17.5	0.26 (1)	10.00			
11-10	0 / 1654	-17.5 -17.5	0.36 (1)	10.00			
10-9	0 / 0	-17.5 -17.5	0.19 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.02")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (1.02")
CALCULATED VERT. DEFL.(TL) = L/999 (0.16")

CSI: TC=0.56/1.00 (1-2:1), BC=0.37/1.00 (13-14:1), WB=0.38/1.00 (1-14:1), SSI=0.22/1.00 (1-2:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

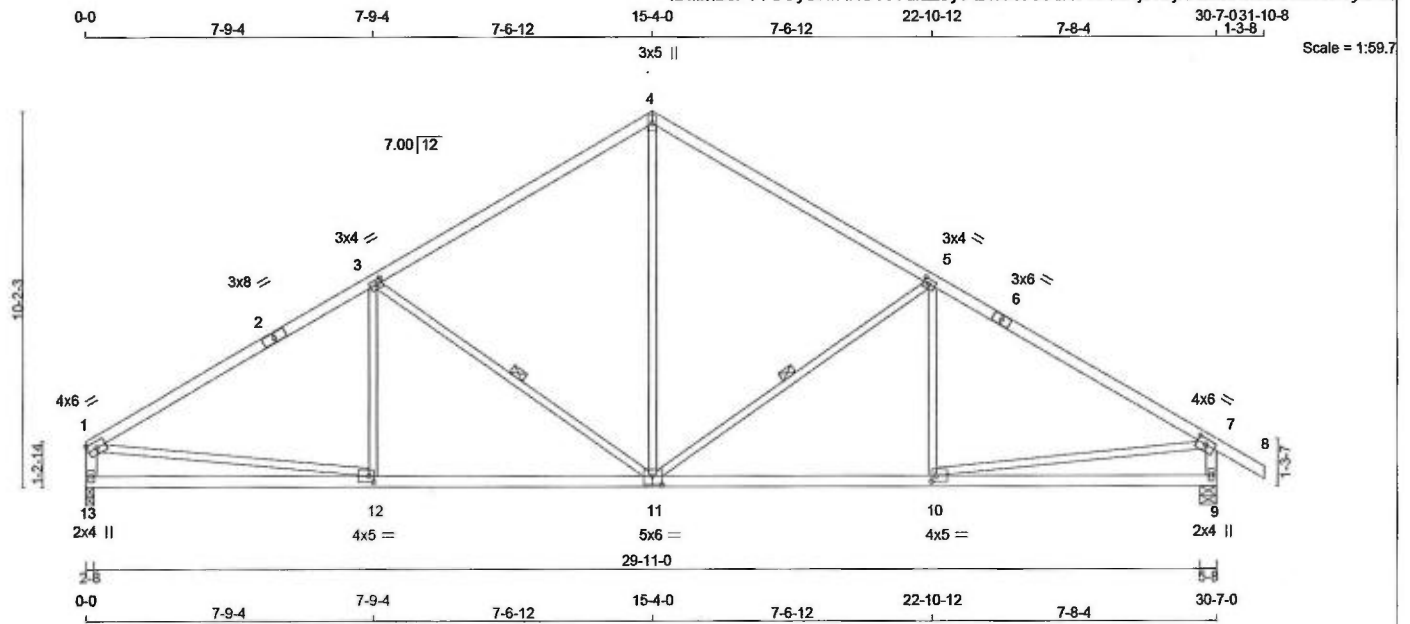
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (4) (INPUT = 0.90)
JSI METAL= 0.52 (7) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
1 - 2	2x4	DRY	No.2	SPF	
2 - 4	2x4	DRY	No.2	SPF	
4 - 6	2x4	DRY	No.2	SPF	
6 - 8	2x4	DRY	No.2	SPF	
13 - 1	2x4	DRY	No.2	SPF	
9 - 7	2x4	DRY	No.2	SPF	
13 - 11	2x4	DRY	No.2	SPF	
11 - 9	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
1	TMW-I	MT20	4.0	6.0		Edge
2	TS-I	MT20	3.0	8.0		
3	TMWV-I	MT20	3.0	4.0	1.50	1.75
4	TTW+p	MT20	3.0	5.0		
5	TMWV-I	MT20	3.0	4.0	1.50	1.75
6	TS-I	MT20	3.0	6.0		
7	TMWV-I	MT20	4.0	6.0	1.75	3.00
9	BMV1+p	MT20	2.0	4.0		
10	BMWV-I	MT20	4.0	5.0	2.00	1.50
11	BSWVW-I	MT20	5.0	6.0	3.00	3.00
12	BMWV-I	MT20	4.0	5.0	2.00	1.50
13	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
13	1451	0	1451	0	2-8	2-8
9	1556	0	1556	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS		PERM. LIVE	WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE					
13	1018	712 / 0	0 / 0	0 / 0	0 / 0	0 / 0	306 / 0	0 / 0
9	1090	776 / 0	0 / 0	0 / 0	0 / 0	0 / 0	314 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 13, 9

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.99 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF 3-11, 5-11. DBS = 20-0-0. CBF = 81 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
1-2	-1886 / 0	-77.4	-77.4 0.77 (1)	12-3	-48 / 113	0.04 (4)	
2-3	-1886 / 0	-77.4	-77.4 0.77 (1)	3-11	-651 / 0	0.40 (1)	
3-4	-1347 / 0	-77.4	-77.4 0.67 (1)	4-6	0 / 881	0.20 (1)	
4-5	-1347 / 0	-77.4	-77.4 0.67 (1)	11-5	-637 / 0	0.39 (1)	
5-6	-1872 / 0	-77.4	-77.4 0.75 (1)	4-3	-60 / 109	0.04 (4)	
6-7	-1872 / 0	-77.4	-77.4 0.75 (1)	1-12	0 / 1672	0.38 (1)	
7-8	0 / 127	-77.4	-77.4 0.10 (1)	10-7	0 / 1661	0.37 (1)	
13-1	-1395 / 0	0.0	0.0 0.14 (1)				
9-7	-1501 / 0	0.0	0.0 0.15 (1)				
13-12	0 / 0	-17.5	-17.5 0.25 (4)	10-10			
12-11	0 / 1660	-17.5	-17.5 0.40 (1)	10-10			
11-10	0 / 1648	-17.5	-17.5 0.39 (1)	10-10			
10-9	0 / 0	-17.5	-17.5 0.25 (4)	10-10			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.02")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (1.02")
CALCULATED VERT. DEFL.(TL) = L/999 (0.17")

CSI: TC=0.77/1.00 (1-3:1), BC=0.40/1.00 (11-12:1), WB=0.40/1.00 (3-11:1), SSI=0.25/1.00 (1-3:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

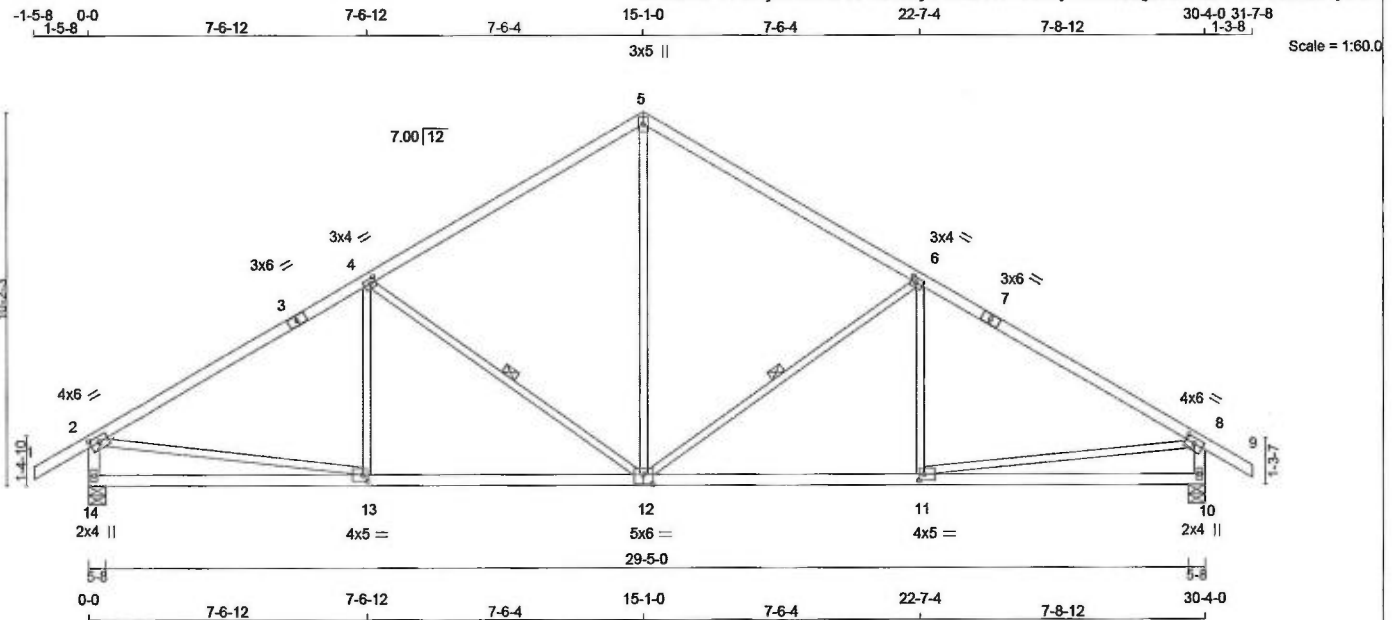
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (1) (INPUT = 0.90)
JSI METAL= 0.52 (7) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 2 X 123 = 247 lb
[M][F]

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
1 - 3	2x4	DRY	No.2
3 - 5	2x4	DRY	No.2
5 - 7	2x4	DRY	No.2
7 - 9	2x4	DRY	No.2
14 - 2	2x4	DRY	No.2
10 - 8	2x4	DRY	No.2
14 - 12	2x4	DRY	No.2
12 - 10	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-I	MT20	4.0	6.0	1.75	3.00
3	TS-I	MT20	3.0	6.0		
4	TMWW-I	MT20	3.0	4.0	1.50	1.75
5	TTW+p	MT20	3.0	5.0		
6	TMWW-I	MT20	3.0	4.0	1.50	1.75
7	TS-I	MT20	3.0	6.0		
8	TMVW-I	MT20	4.0	6.0	1.75	3.00
10	BMV1+p	MT20	2.0	4.0		
11	BMWW-I	MT20	4.0	5.0	2.00	1.50
12	BSWWWW-I	MT20	5.0	6.0	3.00	3.00
13	BMWW-I	MT20	4.0	5.0	2.00	1.50
14	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
JT	1557	0	1557	0	5-8	5-8
14	1557	0	1557	0	5-8	5-8
10	1544	0	1544	0	5-8	5-8

UNFACTORED REACTIONS							
1ST LCASE	MAX./MIN. COMPONENT REACTIONS						
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	1090	778 / 0	0 / 0	0 / 0	0 / 0	313 / 0	0 / 0
10	1081	770 / 0	0 / 0	0 / 0	0 / 0	312 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 14, 10

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.04 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF 4-12, 6-12. DBS = 20-0-0. CBF = 80 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
1-2	0 / 30	-77.4	-77.4 0.13 (1)	10-00	13-4	-81 / 101	0.04 (1)
2-3	-1822 / 0	-77.4	-77.4 0.72 (1)	4.11	4-12	-608 / 0	0.37 (1)
3-4	-1822 / 0	-77.4	-77.4 0.72 (1)	4.11	12-5	0 / 861	0.19 (1)
4-5	-1325 / 0	-77.4	-77.4 0.65 (1)	4.74	12-6	-639 / 0	0.39 (1)
5-6	-1326 / 0	-77.4	-77.4 0.66 (1)	4.71	11-6	-56 / 110	0.04 (4)
6-7	-1851 / 0	-77.4	-77.4 0.75 (1)	4.04	2-13	0 / 1621	0.36 (1)
7-8	-1851 / 0	-77.4	-77.4 0.75 (1)	4.04	11-8	0 / 1642	0.37 (1)
8-9	0 / 27	-77.4	-77.4 0.10 (1)	10.00			
14-2	-1503 / 0	0.0	0.0 0.15 (1)	6.70			
10-8	-1489 / 0	0.0	0.0 0.15 (1)	6.72			
14-13	0 / 0	-17.5	-17.5 0.24 (4)	10.00			
13-12	0 / 1605	-17.5	-17.5 0.38 (1)	10.00			
12-11	0 / 1630	-17.5	-17.5 0.39 (1)	10.00			
11-10	0 / 0	-17.5	-17.5 0.25 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.01")
CALCULATED VERT. DEFL.(LL)= L/999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (1.01")
CALCULATED VERT. DEFL.(TL)= L/999 (0.16")

CSI: TC=0.75/1.00 (6-8:1), BC=0.39/1.00 (11-12:1), WB=0.39/1.00 (6-12:1), SSI=0.25/1.00 (6-8:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP(DRY)	SHEAR	SECTION (PSI)	PLI	PLI
MT20	618	354	1667	822	2284

PLATE PLACEMENT TOL. = 0.250 inches

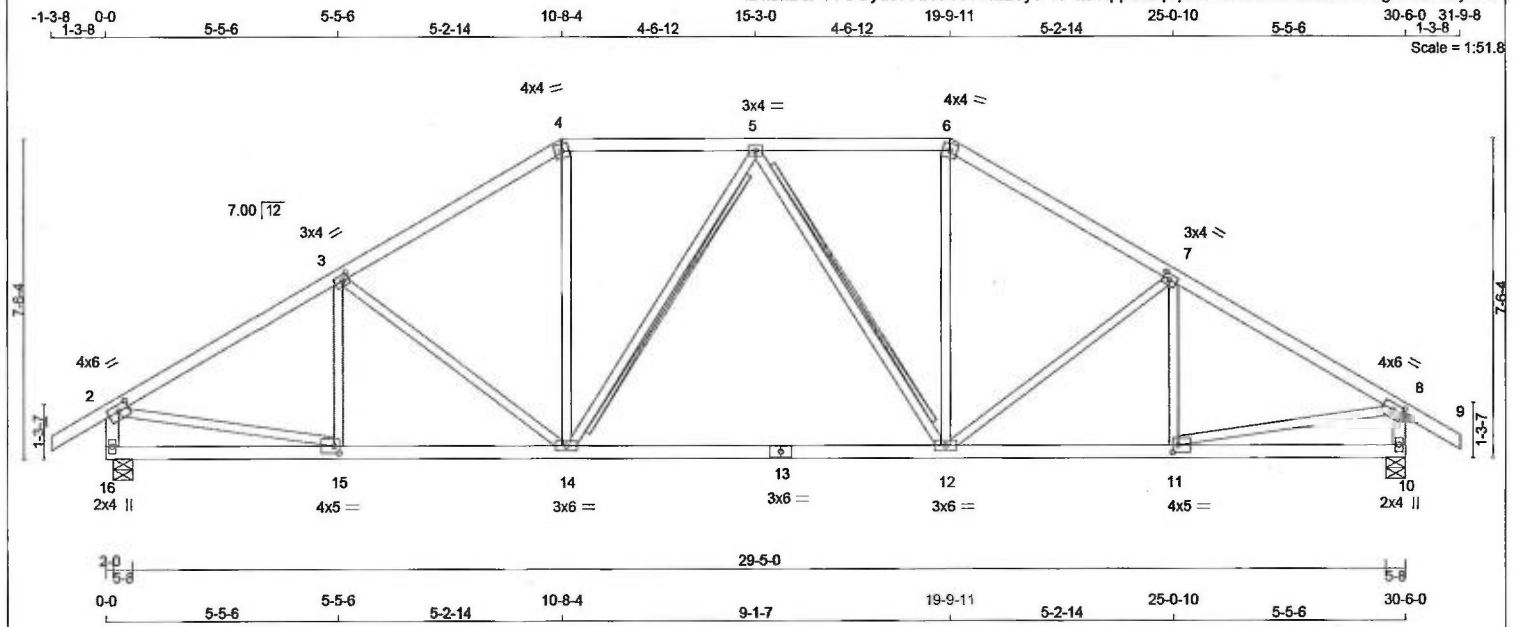
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (12) (INPUT = 0.90)
JSI METAL= 0.51 (8) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 129 lb

LUMBER				DESCR.			
N. L. G. A. RULES							
CHORDS	SIZE	DRY	LUMBER				
1 - 4	2x4	DRY	No.2	SPF			
4 - 6	2x4	DRY	No.2	SPF			
6 - 9	2x4	DRY	No.2	SPF			
16 - 2	2x4	DRY	No.2	SPF			
10 - 8	2x4	DRY	No.2	SPF			
16 - 13	2x4	DRY	No.2	SPF			
13 - 10	2x4	DRY	No.2	SPF			
ALL WEBS	2x3	DRY	No.2	SPF			
EXCEPT							
DRY: SEASONED LUMBER.							

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-I	MT20	4.0	6.0	1.75	3.00
3	TMVW-I	MT20	3.0	4.0	1.50	1.75
4	TTW-m	MT20	4.0	4.0		
5	TMVW-I	MT20	3.0	4.0		
6	TTW-m	MT20	4.0	4.0		
7	TMVW-I	MT20	3.0	4.0	1.50	1.75
8	TMVW-I	MT20	4.0	6.0	1.75	3.00
10	BMV1+p	MT20	2.0	4.0		
11	BMVW-I	MT20	4.0	5.0	2.00	1.50
12	BMVW-I	MT20	3.0	6.0		
13	BS-I	MT20	3.0	6.0		
14	BMVW-I	MT20	3.0	6.0		
15	BMVW-I	MT20	4.0	5.0	2.00	1.50
16	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD IN-SX
	VERT	HORZ	DOWN	HORZ		
16	1552	0	1552	0	5-8	5-8
10	1552	0	1552	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
16	1087	774 / 0	0 / 0	0 / 0	0 / 0	313 / 0	0 / 0
10	1087	774 / 0	0 / 0	0 / 0	0 / 0	313 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 16, 10

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.60 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT 5-14, 5-12

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO	CSI (LC)	FR-TO		FROM TO	CSI (LC)
1-2	0 / 27	-77.4 -77.4	0.10 (1)	10.00	15-3	-205 / 16	0.06 (1)
2-3	-1860 / 0	-77.4 -77.4	0.34 (1)	4.60	3-14	-287 / 0	0.24 (1)
3-4	-1648 / 0	-77.4 -77.4	0.32 (1)	4.85	14-4	0 / 528	0.12 (1)
4-5	-1409 / 0	-77.4 -77.4	0.22 (1)	5.27	14-5	-252 / 0	0.14 (1)
5-6	-1409 / 0	-77.4 -77.4	0.22 (1)	5.27	5-12	-251 / 0	0.14 (1)
6-7	-1649 / 0	-77.4 -77.4	0.32 (1)	4.85	12-6	0 / 528	0.12 (1)
7-8	-1861 / 0	-77.4 -77.4	0.34 (1)	4.60	12-7	-288 / 0	0.24 (1)
8-9	0 / 27	-77.4 -77.4	0.10 (1)	10.00	11-7	-205 / 16	0.06 (1)
16-2	-1509 / 0	0.0 0.0	0.15 (1)	6.68	2-15	0 / 1653	0.37 (1)
10-8	-1509 / 0	0.0 0.0	0.15 (1)	6.68	11-8	0 / 1654	0.37 (1)
16-15	0 / 0	-17.5 -17.5	0.11 (4)	10.00			
15-14	0 / 1628	-17.5 -17.5	0.39 (1)	10.00			
14-13	0 / 1541	-17.5 -17.5	0.37 (1)	10.00			
13-12	0 / 1541	-17.5 -17.5	0.37 (1)	10.00			
12-11	0 / 1628	-17.5 -17.5	0.39 (1)	10.00			
11-10	0 / 0	-17.5 -17.5	0.11 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
	DL = 3.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.02")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (1.02")
CALCULATED VERT. DEFL.(TL) = L/999 (0.22")

CSI: TC=0.34/1.00 (7-8:1), BC=0.39/1.00 (11-12:1), WB=0.37/1.00 (8-11:1), SSI=0.17/1.00 (7-8:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

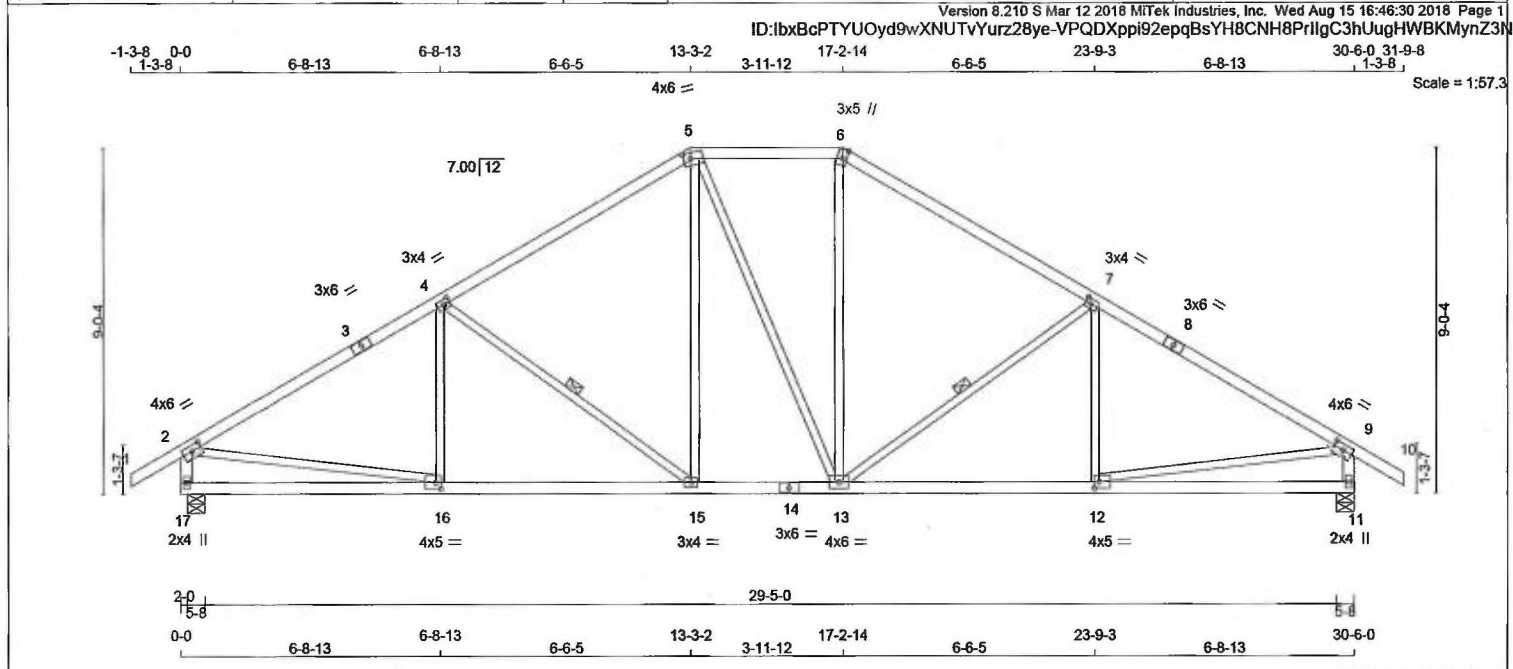
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (15) (INPUT = 0.90)
JSI METAL= 0.56 (13) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.	BEARINGS	FACTORED	MAXIMUM FACTORED
1 - 3	2x4	DRY	No.2	SPF	VERT	GROSS REACTION	INPUT
3 - 5	2x4	DRY	No.2	SPF	HORZ	DOWN	BRG
5 - 6	2x4	DRY	No.2	SPF	UPLIFT	IN-SX	IN-SX
6 - 8	2x4	DRY	No.2	SPF	17	1552 0	5-8
8 - 10	2x4	DRY	No.2	SPF	11	1552 0	5-8
10 - 11	2x4	DRY	No.2	SPF			
11 - 12	2x4	DRY	No.2	SPF			
12 - 13	2x4	DRY	No.2	SPF			
13 - 14	2x4	DRY	No.2	SPF			
14 - 15	2x4	DRY	No.2	SPF			
15 - 16	2x4	DRY	No.2	SPF			
16 - 17	2x4	DRY	No.2	SPF			
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF			

DRY: SEASONED LUMBER.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 PSF

DL = 3.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.0 PSF

TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014

- CSA 086-09

- TPIC 2011

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-I	MT20	4.0	6.0	1.75	3.00
3	TS-I	MT20	3.0	6.0		
4	TMVW-I	MT20	3.0	4.0	1.50	1.75
5	TTWW-m	MT20	4.0	6.0	1.75	2.25
6	TTW+m	MT20	3.0	5.0	2.75	1.25
7	TMVW-I	MT20	3.0	4.0	1.50	1.75
8	TS-I	MT20	3.0	6.0		
9	TMVW-I	MT20	4.0	6.0	1.75	3.00
11	BMV1+p	MT20	2.0	4.0		
12	BMVW-I	MT20	4.0	5.0	2.00	1.50
13	BMVW-I	MT20	4.0	6.0		
14	BS-I	MT20	3.0	6.0		
15	BMVW-I	MT20	3.0	4.0		
16	BMVW-I	MT20	4.0	5.0	2.00	1.50
17	BMV1+p	MT20	2.0	4.0		

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
1-2	0/27	-77.4 -77.4	0.10 (1)	10.00	16-4	-105/83	0.04 (1)
2-3	-1877/0	-77.4 -77.4	0.55 (1)	4.33	4-15	-502/0	0.23 (1)
3-4	-1877/0	-77.4 -77.4	0.55 (1)	4.33	15-5	0/382	0.09 (1)
4-5	-1473/0	-77.4 -77.4	0.49 (1)	4.82	5-13	0/2	0.00 (1)
5-6	-1249/0	-77.4 -77.4	0.17 (1)	5.57	13-6	0/385	0.09 (1)
6-7	-1474/0	-77.4 -77.4	0.49 (1)	4.82	13-7	-501/0	0.23 (1)
7-8	-1877/0	-77.4 -77.4	0.55 (1)	4.33	12-7	-106/82	0.04 (1)
8-9	-1877/0	-77.4 -77.4	0.55 (1)	4.33	2-16	0/1665	0.37 (1)
9-10	0/27	-77.4 -77.4	0.10 (1)	10.00	12-9	0/1665	0.37 (1)
17-2	-1504/0	0.0 0.0	0.15 (1)	6.69			
11-9	-1504/0	0.0 0.0	0.15 (1)	6.69			
17-16	0/0	-17.5 -17.5	0.20 (4)	10.00			
16-15	0/1648	-17.5 -17.5	0.36 (1)	10.00			
15-14	0/1248	-17.5 -17.5	0.26 (1)	10.00			
14-13	0/1248	-17.5 -17.5	0.26 (1)	10.00			
13-12	0/1648	-17.5 -17.5	0.36 (1)	10.00			
12-11	0/0	-17.5 -17.5	0.20 (4)	10.00			

LOADING

TOTAL LOAD CASES: (4)

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

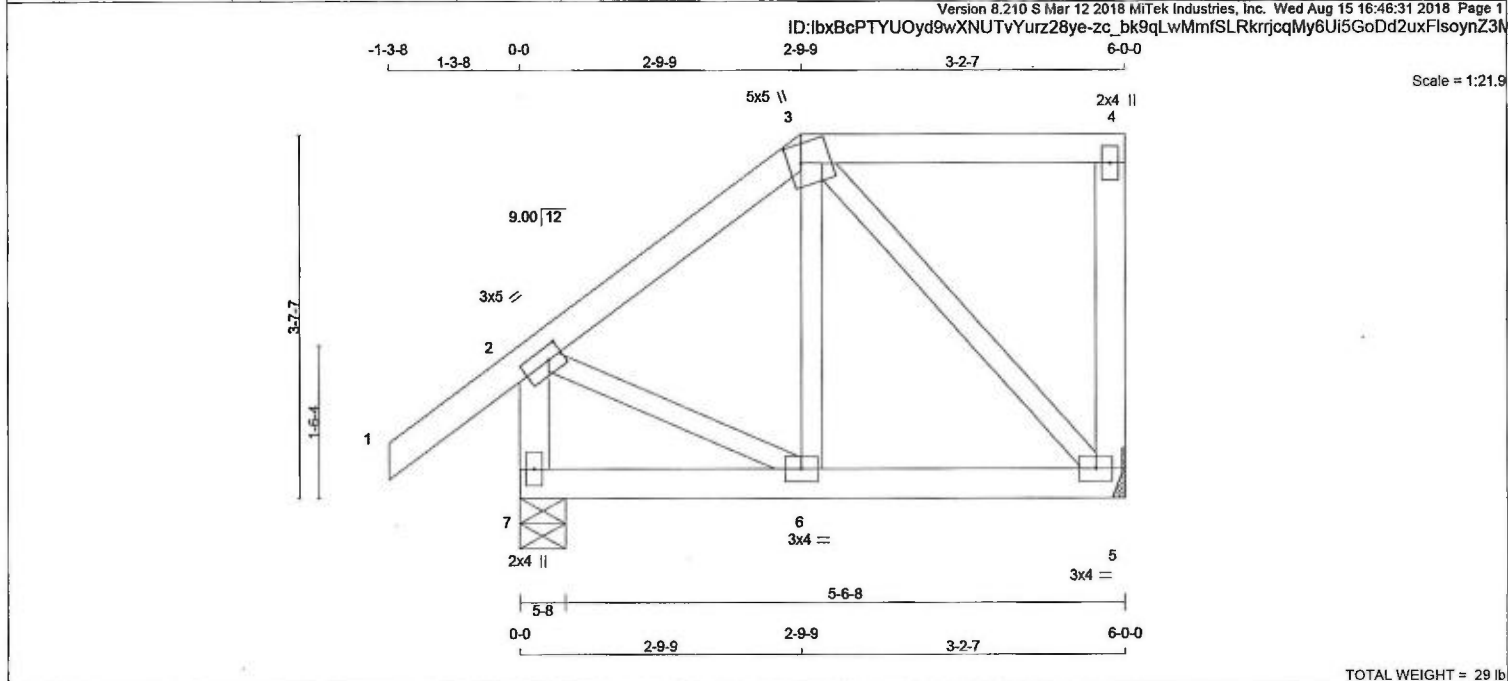
JSI GRIP= 0.89 (6) (INPUT = 0.90)

JSI METAL= 0.51 (9) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
1 - 3	2x4	DRY	No.2
3 - 4	2x4	DRY	No.2
5 - 4	2x4	DRY	No.2
7 - 2	2x4	DRY	No.2
7 - 5	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-t	MT20	3.0	5.0	1.50	1.75
3	TTWW+m	MT20	5.0	5.0	2.25	1.50
4	TMV+p	MT20	2.0	4.0		
5	BMVW1-t	MT20	3.0	4.0		
6	BMVW-t	MT20	3.0	4.0		
7	BMV1+p	MT20	2.0	4.0		

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER							
BEARINGS							
	FACTORED	MAXIMUM FACTORED	INPUT	REQD			
	GROSS REACTION	GROSS REACTION	BRG	BRG			
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
5	285	0	285	0	0	MECHANICAL	
7	391	0	391	0	0	5-8	5-8

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT 5 TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS							
JT	1ST LCASE	MAX./MIN.	COMPONENT	REACTIONS			
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
5	200	140 / 0	0 / 0	0 / 0	0 / 0	60 / 0	0 / 0
7	272	204 / 0	0 / 0	0 / 0	0 / 0	68 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 7

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED UNBRAC (LBS)	LC1 MAX (LC)
FR-TO		FROM	TO	FR-TO		FROM	TO
1-2	0 / 32	-77.4	-77.4 0.11 (1)	10.00	6-3	0 / 54	0.02 (4)
2-3	-168 / 0	-77.4	-77.4 0.10 (1)	6.25	3-5	-193 / 0	0.06 (1)
3-4	0 / 0	-77.4	-77.4 0.14 (1)	10.00	2-6	0 / 145	0.03 (1)
5-4	-124 / 0	0.0	0.0 0.03 (1)	7.81			
7-2	-371 / 0	0.0	0.0 0.04 (1)	7.81			
7-6	0 / 0	-17.5	-17.5 0.04 (4)	10.00			
6-5	0 / 134	-17.5	-17.5 0.05 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.00")

CSI: TC=0.14/1.00 (3-4:1) , BC=0.05/1.00 (5-6:4) , WB=0.06/1.00 (3-5:1) , SSI=0.10/1.00 (3-4:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

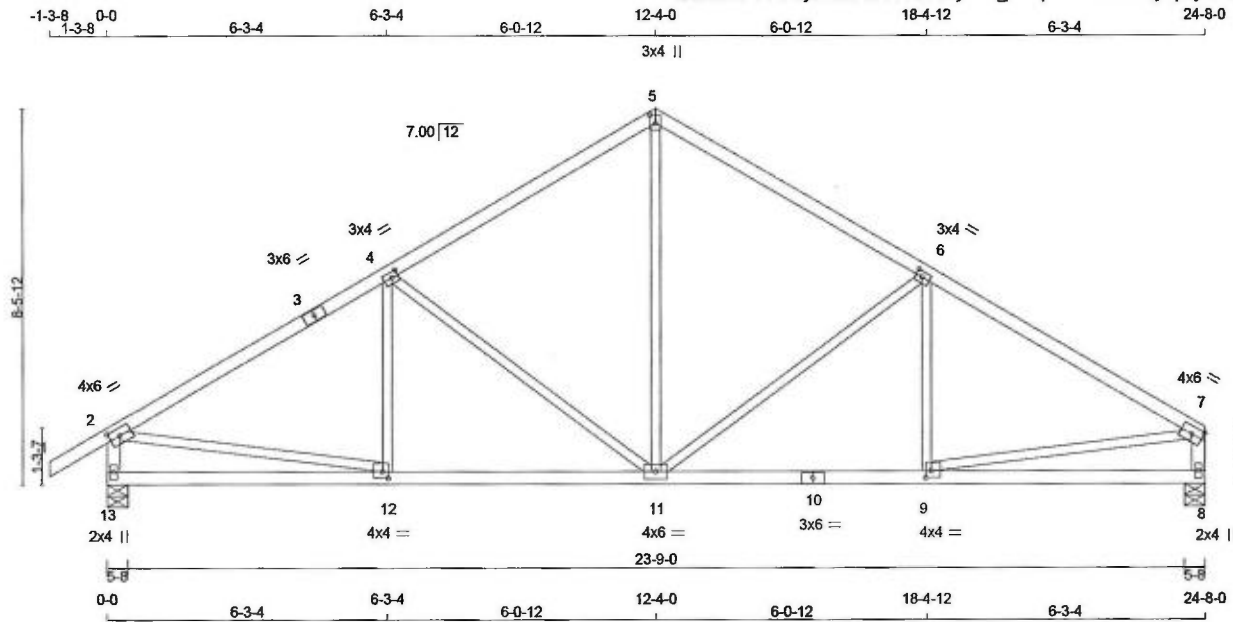
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.31 (2) (INPUT = 0.90)
JSI METAL= 0.09 (2) (INPUT = 1.00)



TOTAL WEIGHT = 2 X 100 = 200 lb [M][F]

CHORDS	SIZE	LUMBER	DESCR.
1 - 3	2x4	DRY No.2	SPF
3 - 5	2x4	DRY No.2	SPF
5 - 7	2x4	DRY No.2	SPF
13 - 2	2x4	DRY No.2	SPF
8 - 7	2x4	DRY No.2	SPF
13 - 10	2x4	DRY No.2	SPF
10 - 8	2x4	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-I	MT20	4.0	6.0	2.00	3.00
3	TS-I	MT20	3.0	6.0		
4	TMWW-I	MT20	3.0	4.0	1.50	1.75
5	TTW+p	MT20	3.0	4.0	2.25	1.50
6	TMWW-I	MT20	3.0	4.0	1.50	1.75
7	TMVW-I	MT20	4.0	6.0		Edge
8	BMV1+p	MT20	2.0	4.0		
9	BMWW-I	MT20	4.0	4.0	1.75	1.50
10	BS-I	MT20	3.0	6.0		
11	BMWW-I	MT20	4.0	6.0		
12	BMWW-I	MT20	4.0	4.0	1.75	1.50
13	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
13	1276	0	1276	0	5-8	5-8
8	1170	0	1170	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
13	893	638 / 0	0 / 0	0 / 0	0 / 0	255 / 0	0 / 0
8	821	574 / 0	0 / 0	0 / 0	0 / 0	247 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 13, 8

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.96 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
1-2	0 / 27	-77.4 -77.4	0.10 (1)	10.00	12-4	-79 / 79 0.03 (1)	
2-3	-1443 / 0	-77.4 -77.4	0.43 (1)	4.96	4-11	-480 / 0 0.56 (1)	
3-4	-1443 / 0	-77.4 -77.4	0.43 (1)	4.96	11-5	0 / 686 0.15 (1)	
4-5	-1059 / 0	-77.4 -77.4	0.40 (1)	5.62	11-6	-480 / 0 0.56 (1)	
5-6	-1059 / 0	-77.4 -77.4	0.40 (1)	5.62	9-6	-78 / 79 0.03 (1)	
6-7	-1443 / 0	-77.4 -77.4	0.43 (1)	4.96	2-12	0 / 1286 0.29 (1)	
13-2	-1230 / 0	0.0 0.0	0.13 (1)	7.23	9-7	0 / 1286 0.29 (1)	
8-7	-1124 / 0	0.0 0.0	0.11 (1)	7.48			
13-12	0 / 0	-17.5 -17.5	0.16 (4)	10.00			
12-11	0 / 1271	-17.5 -17.5	0.28 (1)	10.00			
11-10	0 / 1271	-17.5 -17.5	0.28 (1)	10.00			
10-9	0 / 1271	-17.5 -17.5	0.28 (1)	10.00			
9-8	0 / 0	-17.5 -17.5	0.16 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.82")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL)= L/380 (0.82")
CALCULATED VERT. DEFL.(TL) = L/999 (0.10")

CSI: TC=0.43/1.00 (6-7:1), BC=0.28/1.00 (9-11:1), WB=0.56/1.00 (6-11:1), SSI=0.20/1.00 (6-7:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

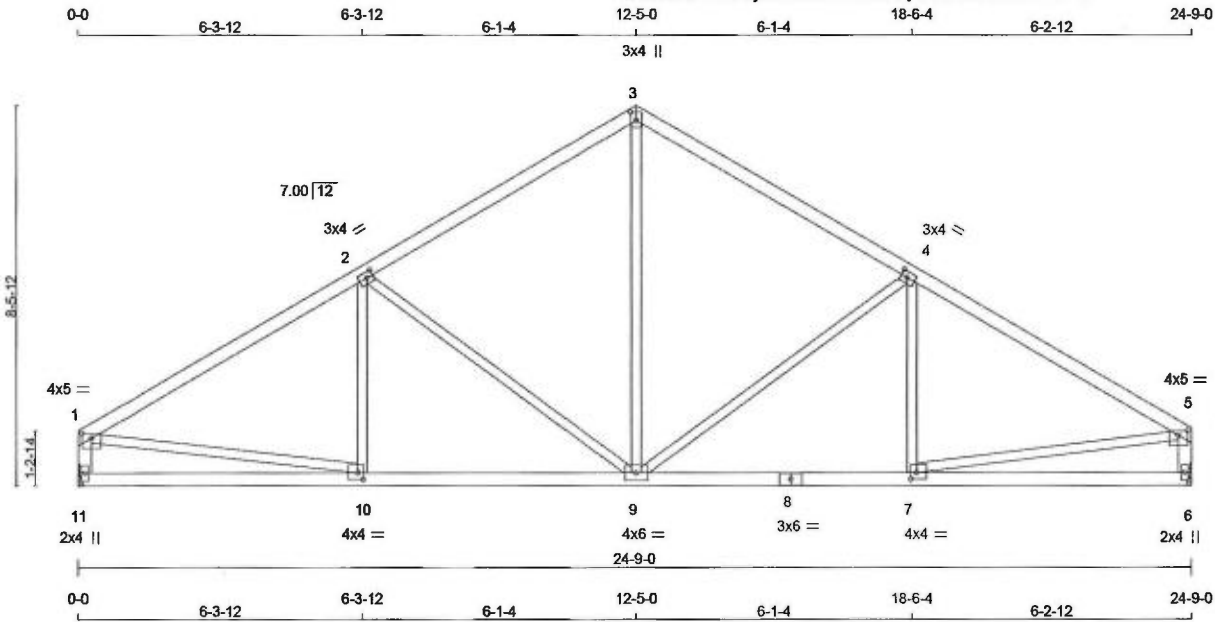
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (5) (INPUT = 0.90)
JSI METAL= 0.48 (12) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 98 lb
(M/F)

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
1 - 3	2x4	DRY	No.2
3 - 5	2x4	DRY	No.2
11 - 1	2x4	DRY	No.2
6 - 5	2x4	DRY	No.2
11 - 8	2x4	DRY	No.2
8 - 6	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
1	TMVW-p	MT20	4.0	5.0	1.25	2.50
2	TMWW-l	MT20	3.0	4.0	1.50	1.75
3	TTW+p	MT20	3.0	4.0	2.25	1.50
4	TMWW-l	MT20	3.0	4.0	1.50	1.75
5	TMVW-p	MT20	4.0	5.0	1.25	2.50
6	BMV1+p	MT20	2.0	4.0		
7	BMWW-l	MT20	4.0	4.0	1.75	1.50
8	BS-l	MT20	3.0	6.0		
9	BMWW-l	MT20	4.0	6.0		
10	BMWW-l	MT20	4.0	4.0	1.75	1.50
11	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
		GROSS REACTION		GROSS REACTION		BRG		BRG	
JT		VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
11		1174	0	1174	0	0	MECHANICAL	MECHANICAL	
6		1174	0	1174	0	0	MECHANICAL	MECHANICAL	

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT 11, 6 TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

		1ST LCASE	MAX/MIN		COMPONENT REACTIONS				
JT		COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
11		824	576 / 0	0 / 0	0 / 0	0 / 0	248 / 0	0 / 0	
6		824	576 / 0	0 / 0	0 / 0	0 / 0	248 / 0	0 / 0	

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.92 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO		FR-TO			
1-2	-1464 / 0	-77.4 -77.4	0.44 (1)	4.92	10-2	-69 / 83	0.03 (4)
2-3	-1067 / 0	-77.4 -77.4	0.40 (1)	5.59	2-9	-493 / 0	0.58 (1)
3-4	-1066 / 0	-77.4 -77.4	0.40 (1)	5.60	9-3	0 / 690	0.16 (1)
4-5	-1451 / 0	-77.4 -77.4	0.43 (1)	4.95	9-4	-479 / 0	0.57 (1)
11-1	-1128 / 0	0.0 0.0	0.11 (1)	7.48	7-4	-81 / 79	0.03 (1)
6-5	-1128 / 0	0.0 0.0	0.11 (1)	7.48	1-10	0 / 1303	0.29 (1)
11-10	0 / 0	-17.5 -17.5	0.16 (4)	10.00	7-5	0 / 1293	0.29 (1)
10-9	0 / 1289	-17.5 -17.5	0.29 (1)	10.00			
9-8	0 / 1278	-17.5 -17.5	0.28 (1)	10.00			
8-7	0 / 1278	-17.5 -17.5	0.28 (1)	10.00			
7-6	0 / 0	-17.5 -17.5	0.16 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.83")
CALCULATED VERT. DEFL.(LL)= L/999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.83")
CALCULATED VERT. DEFL.(TL)= L/999 (0.10")

CSI: TC=0.44/1.00 (1-2:1), BC=0.29/1.00 (9-10:1), WB=0.58/1.00 (2-9:1), SSI=0.20/1.00 (1-2:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

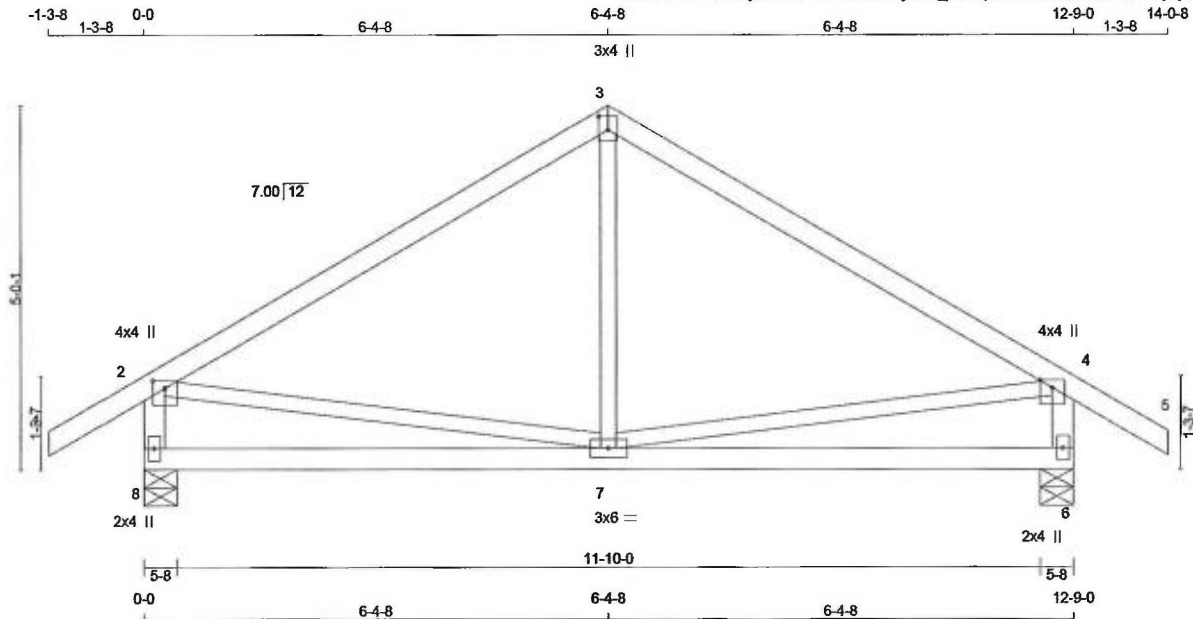
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (1) (INPUT = 0.90)
JSI METAL= 0.48 (10) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 50 lb
[M/JF]

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
1 - 3	2x4	DRY	No.2
3 - 5	2x4	DRY	No.2
8 - 2	2x4	DRY	No.2
6 - 4	2x4	DRY	No.2
8 - 6	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY; SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW+p	MT20	4.0	4.0	1.25	2.00
3	TTW+p	MT20	3.0	4.0	2.25	1.50
4	TMVW+p	MT20	4.0	4.0	1.25	2.00
6	BMV1+p	MT20	2.0	4.0		
7	BMVWW-t	MT20	3.0	6.0		
8	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	BRG
8	710	0	710	0	0	5-8	5-8	5-8	
6	710	0	710	0	0	5-8	5-8	5-8	

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
8	COMBINED	360 / 0	0 / 0	0 / 0	0 / 0	0 / 0	136 / 0	0 / 0
6	COMBINED	360 / 0	0 / 0	0 / 0	0 / 0	0 / 0	136 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 8, 6

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO		FROM TO			FR-TO		
1-2	0 / 27	-77.4	-77.4 0.10 (1)	10.00	7-3	0 / 112	0.04 (4)
2-3	-496 / 0	-77.4	-77.4 0.41 (1)	6.25	2-7	0 / 434	0.10 (1)
3-4	-496 / 0	-77.4	-77.4 0.41 (1)	6.25	7-4	0 / 434	0.10 (1)
4-5	0 / 27	-77.4	-77.4 0.10 (1)	10.00			
8-2	-668 / 0	0.0	0.0 0.07 (1)	7.81			
6-4	-668 / 0	0.0	0.0 0.07 (1)	7.81			
8-7	0 / 0	-17.5	-17.5 0.20 (4)	10.00			
7-6	0 / 0	-17.5	-17.5 0.20 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
DL = 3.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.0	PSF	
TOTAL LOAD = 33.3	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.41/1.00 (3-4:1), BC=0.20/1.00 (7-8:4),
WB=0.10/1.00 (2-7:1), SSI=0.17/1.00 (3-4:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
MT20	618	354	1667
	822	2284	1658

PLATE PLACEMENT TOL. = 0.250 inches

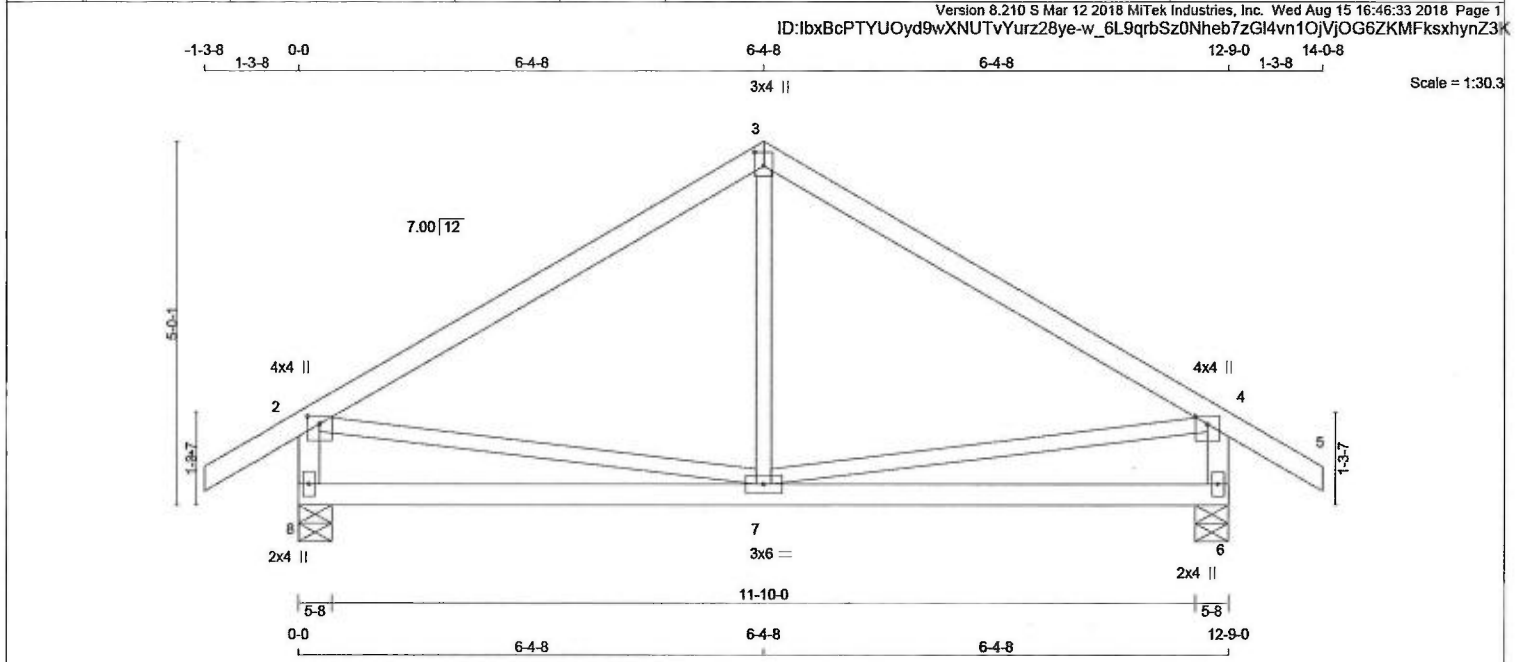
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (7) (INPUT = 0.90)
JSI METAL= 0.16 (2) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
1 - 3	2x4	DRY No.2	SPF
3 - 5	2x4	DRY No.2	SPF
8 - 2	2x4	DRY No.2	SPF
6 - 4	2x4	DRY No.2	SPF
8 - 6	2x4	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW+p	MT20	4.0	4.0	1.25	2.00
3	TTW+p	MT20	3.0	4.0	2.25	1.50
4	TMVW+p	MT20	4.0	4.0	1.25	2.00
6	BMV1+p	MT20	2.0	4.0		
7	BMVWW-t	MT20	3.0	6.0		
8	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
8	710	0	710	0	5-8	5-8
6	710	0	710	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
8	496	360 / 0	0 / 0	0 / 0	0 / 0	136 / 0	0 / 0
6	496	360 / 0	0 / 0	0 / 0	0 / 0	136 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 8, 6

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX LC1 (LC)
FR-TO		FROM TO			FR-TO		
1-2	0 / 27	-77.4	-77.4 0.10 (1)	10.00	7-3	0 / 112	0.04 (4)
2-3	-496 / 0	-77.4	-77.4 0.41 (1)	6.25	2-7	0 / 434	0.10 (1)
3-4	-496 / 0	-77.4	-77.4 0.41 (1)	6.25	7-4	0 / 434	0.10 (1)
4-5	0 / 27	-77.4	-77.4 0.10 (1)	10.00			
8-2	-668 / 0	0.0	0.0 0.07 (1)	7.81			
6-4	-668 / 0	0.0	0.0 0.07 (1)	7.81			
8-7	0 / 0	-17.5	-17.5 0.20 (4)	10.00			
7-6	0 / 0	-17.5	-17.5 0.20 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	PSF
	23.3	PSF
BOT CH.	LL	PSF
	3.0	PSF
	0.0	PSF
	7.0	PSF
TOTAL LOAD	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

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DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20 618 354	1667 822	2264 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (7) (INPUT = 0.90)
JSI METAL= 0.16 (2) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-1. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



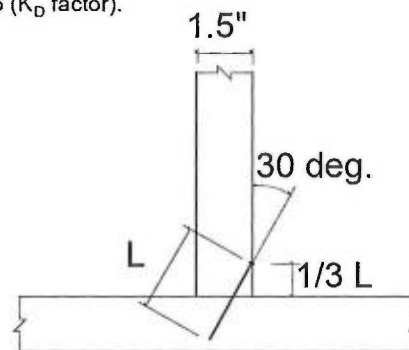
BEARING ANCHORAGE BY TOE-NAILS FOR LATERAL CAPACITY

B37579H1

NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL LATERAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	132	147
	3.25	0.144	132	147
	3.50	0.160	159	177

NOTES:

1. Rafter and ceiling members may be anchored to top and bottom chords of girder truss by toe-nailing rafter and ceiling members to girder chords provided the reaction does not exceed the lateral capacities in the table. Hangers (specified by others) are required for reactions higher than the maximum toe-nail capacity. Reactions are based on factored loads.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-09, section 10.9.4.1.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in tables below.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See next page for nailing on bearing plate).
7. For loads due to **wind** the nail lateral capacity in this table may be multiplied by 1.15 (K_D factor).
8. Lumber must be dry (< 19% moisture content) at the time of nail installation.
9. Nail values in this table comply with CSA O86-09, section 10.9.4
10. This design is not valid after April 30, 2019.



TOE-NAIL INSTALLATION

Nail type	Common wire	Common spiral	Common wire
Nail dia. (in)	0.160	0.152	0.144
	(3.5" nail)		(3" and 3.25" nail)
LUMBER SIZE	MAXIMUM NUMBER OF TOE-NAILS		
2X4 SPF	2	2	3
2X4 D. Fir	2	2	2
2X6 SPF	4	4	4
2X6 D. Fir	3	3	3



MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

PEO
Certificate No. 10889485



April 26, 2017

BEARING ANCHORAGE BY TOE-NAILS FOR WIND LOADING

B37579H2

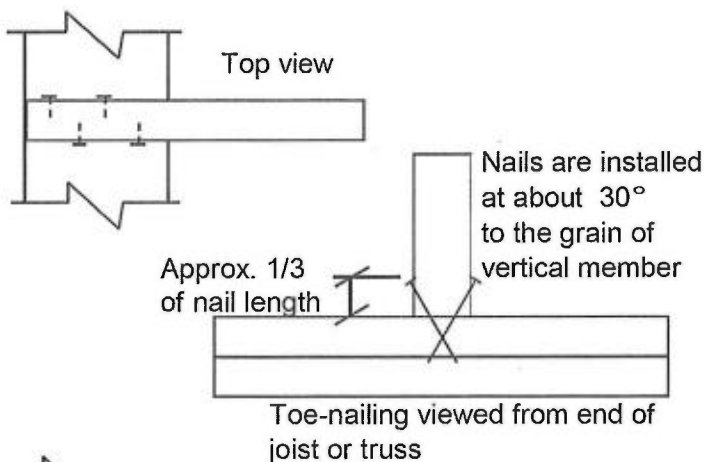
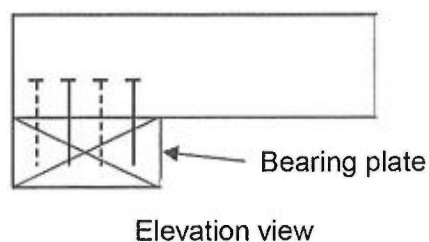
NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL WITHDRAWAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	30	42
	3.25	0.144	32	45
	3.50	0.160	38	52

Note: If using truss with D. Fir lumber and S-P-F bearing plate, use values in table for S-P-F.

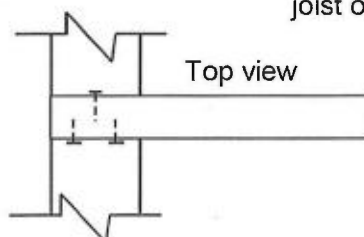
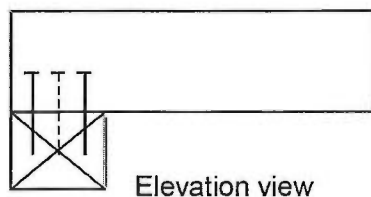
NOTES:

1. Truss chord, rafter, or ceiling members may be anchored to bearing plate by toe-nails, provided that the actual factored uplift force due to **wind or earthquake** load does not exceed the withdrawal capacities in the table. Hangers (specified by others) are required for uplift forces that are higher than the maximum toe-nail withdrawal capacity.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-09, section 10.9.5.2.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in table above.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See drawing on detail B37579H1).
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-09, section 10.9.5
9. This design is not valid after April 30, 2019

Toe-nailing on 2x6 Bearing Plate



Toe-nailing on 2x4 Bearing Plate



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April 26, 2017