Layout Name MILLWOOD 2-ELEV 3 Design Method LSD Description

Created

June 25, 2018 Builder

GREENPARK

Sales Rep RM Designer

RO

Shipping Project

Canada

K2H7V1 905-642-4400

Job Path

Design Method

Floor Loads

Live

Dead

Builder's Project

14 Anderson Blvd

Stouffville, Ontario

Kott Lumber Company

S:\CUSTOMERS\GREENPARK

MINNISALE HOMES\MODELS \MILLWOOD 2\FLOORS\ELEV 3

Building Code NBCC 2010 / OBC

LSD

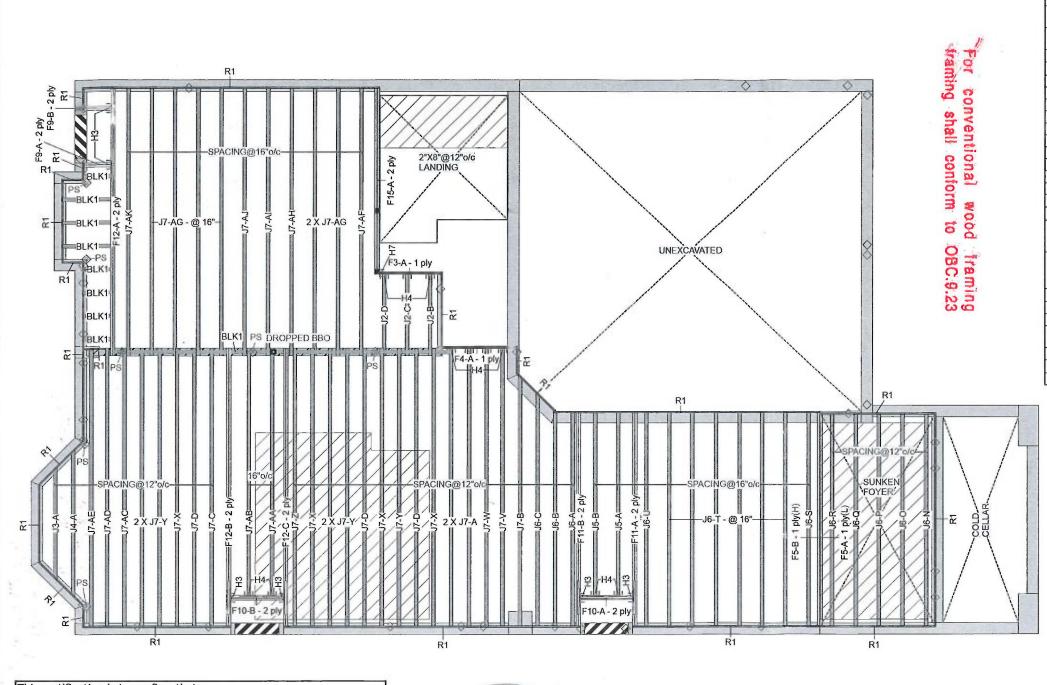
2012

40

15

\MILLWOOD 2-ELEV 3.isl **Ground Floor**

Qty Plies Pcs Length



This certification is to confirm that:

Version 18.40.162 Powered by iStruct™

Ground Floor

1. The loads used in the calculation of the attached approved components conform to the floor assembly shown on this layout. 2. The floor joists comply with the Nascor span table for the loads and spacing shown on this layout.

The floor system must be assembled in accordance to the Nascor Specifier Guide. Multi-ply members must be attached together as per the included multiple member connection detail.

All other components and structural elements supporting the floor system such as beams, walls, columns and foundation walls and footings including anchorage of components and bracing for lateral stability are the responsibility of others.



This layout is to be used as an installation guide only. It is meant to be used in conjunction with the architectural and structural drawings, not to replace them

1. OBC 2012 O.Reg 332/12 as amende

2. Nascor CCMC - 13535-R

3. LVL CCMC -14056-R

4. CAN/CSA-O86-09

5. CCMC -12787-R APA PR-L310(C)

Lapei	Descri	puon	AAIG	am !	De	pui	,	-ciy	Files	PCS	Lengin
F5	Forex 2.0E-3000Fb LVL		1.	.75	11.8	875				2	14-0-0
F15	Forex 2.0E-3	000Fb LVL	1.	.75	11.8	875		1	2	2	12-0-0
F4	Forex 2.0E-30	000Fb LVL		.75	11.8					1	6-0-0
F3		000Fb LVL	1.	.75	11.8	375				1	4-0-0
,	Flush)										
Label	Descri	ption	Wid		De	•	(Qty	Plies	Pcs	Length
F12	NJ			1.5	11.8			3	2	6	16-0-0
F11	NJ			1.5	11.8	_		2	2	4	14-0-0
F10	NJ			1.5	11.8			2	2	4	4-0-0
F9	NJ			1.5	11.8			2	2	4	2-0-0
J7	NJH			2.5	11.8					35	16-0-0
J6	NJH			2.5	11.8					16	14-0-0
J5	NJH			2.5	11.8	_				2	12-0-0
J4	NJH			2.5	11.8					1	10-0-0
J3	NJH		2	2.5	11.8	375				1	8-0-0
J2	2 NJH		2	2.5	11.8	875				3	6-0-0
Rim Bo	ard										
Label	Descri	iption	Wid	ith	De	pth	(Qty	Plies	Pcs	Length
R1		d Rimboard 125 X	1.13	25	11.8	375				12	12
langer											
						S	- 2	Bea	am/Girder		ported ember
Label	Pcs	Description	n	S	kew	Slo	ре	fa	asteners	fas	teners
НЗ	6	LT2-151188	,					4 1	10dx1 1/2	2 10	ldx1 1/2
H4	11	LT251188		\Box				4 1	10dx1 1/2	2 10	dx1 1/2
H7	1	HUCQ1.81/ SDS	9-								
Blockin	g						_				
Label	Descri	iption	Wid	Ith	De	pth	(Qty	Plies	Pcs	Length
BLK1	NJH		2	2.5	11.8			inFt		Varies	29-0-0
Ir	NOTE	G.									
1	1. Frame	er to verify di le joist only r									

Width Depth 1.75 11.875

Ground Floor

LVL/LSL (Flush)

Label Description

another member using a face-mounted hanger. Install 2x4 blocking @ 24"o/c under parallel non-load bearing walls.

4. Install single-ply flush window header along inside face of rimboard/rimioist

5. Refer to Nascor specifier guide for installation works. . Squash blocks recommended to be installed at end bearing on

all first level joists which support loading from above exceeding two levels floor or roof.

Load transfer blocks to be installed under all point loads.
 It shall be the frame's responsibility that floor joists and beams are fastened as per the hanger manufacturer's standards.

Refer to Multiple Member Connection Detail to ply to ply nailing or

Rim parallel to joists: 1-1/8" rimboard with 2"x4" block (1/16" longer than rim depth @ 16"o/c). All other components and structural elements supporting the floor system such as beams, walls, columns, and foundation walls, and footings including anchorage of components and bracing for lateral stability are the responsibility of Others.

Hatch are represents ceramic tiled floor with an additional dead load of 5 PSF

The framing shown on this layout may deviate from the arg and structural drawings. Project Engineer to review and apporte the deviprior to construction.

ARCHITECTURAL DRAWINGS:

JARDIN DESIGN GROUP INC 64 Jardin Dr. Suite 3A Date: Rev. 1, 4/26/2018 Project No: 2645 Model: Millwood 2, Elevation 1

Legend

0

Point Load Support Load from Above Wall Opening

NJH 11.875

Norbord Rimboard Plus 1.125 X 11.875 NJ 11.875

Forex 2.0E-3000Fb LVL 1.75 X 11.875

1096855

Deflection Joist 480 LL Span L/ TL Span L/ 360 480 LL Cant 2L/ 360 TL Cant 2L/ Deflection Girder LL Span L/ 360 TL Span L/ 240 480 LL Cant 2L/ TL Cant 2L/ 240 Decking SPF Plywood Deck Thickness Fastener Nailed & Glued

Vibration

II work shall conform to tuilding Code O. Reg. 332/12 the Onta Second Floor

Layout Name

LSD

Live

Dead

Deflection Joist

LL Span L/

TL Span L/

LL Cant 2L/

TL Cant 2L/

LL Span L/

TL Span L/

LL Cant 2L/ TL Cant 2L/

Decking

Thickness

Fastener

Vibration

Deck

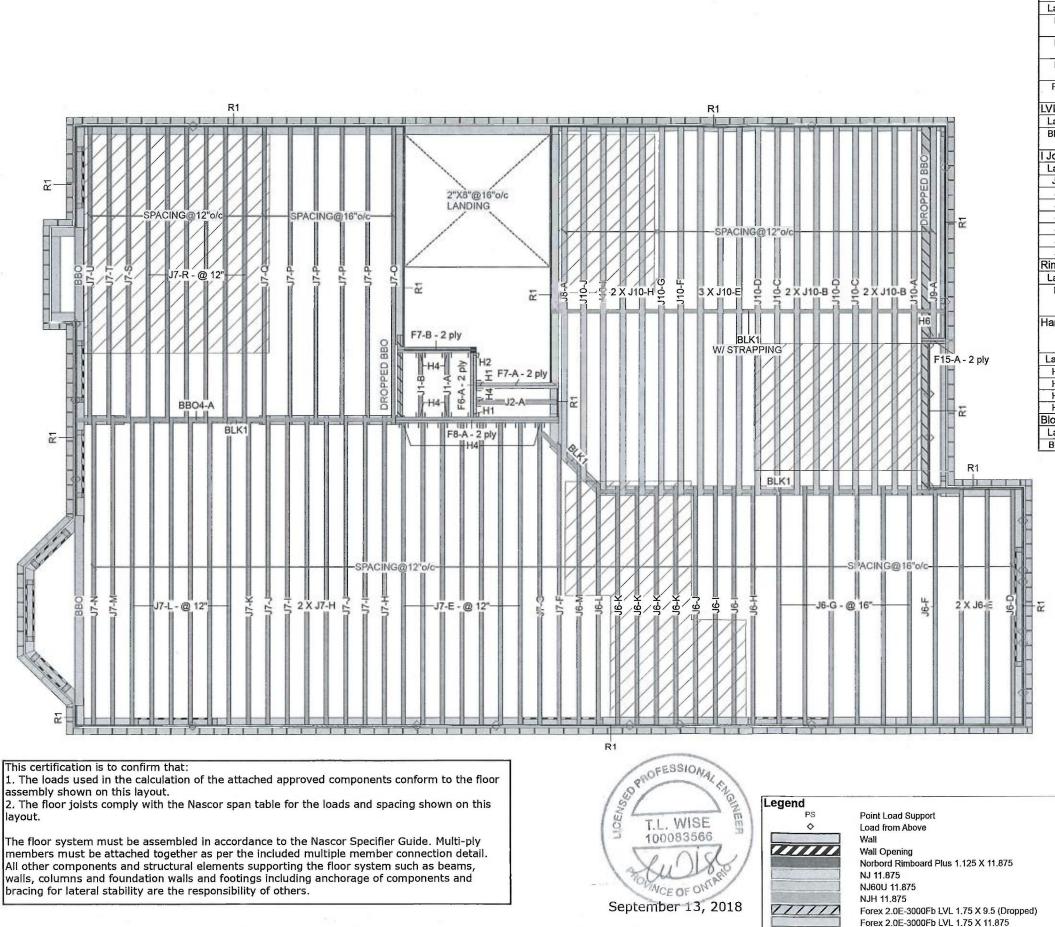
Deflection Girder

Created

June 25, 2018

GREENPARK

MILLWOOD 2-ELEV 3



Second Floor LVL/LSL (Flush) Pcs Length Qty Plies Label Description Width Depth 11.875 2 10-0-0 F8 1.75 2 Forex 2.0E-3000Fb LVL 6-0-0 F7 1.75 11.875 Forex 2 2 4 2.0E-3000Fb LVL Design Method F6 1.75 11.875 4-0-0 Forex 2.0E-3000Fb LVL F15 Forex 1.75 11.875 2-0-0 Description 2.0E-3000Fb LVL LVL/LSL (Dropped) Label Description Width Depth Qty Plies Pcs Length 1.75 9.5 BBO4 Forex 2 2 8-0-0 1 Builder 2.0E-3000Fb LVL Joist (Flush) Label Description Width Depth Qty Plies Pcs Length Sales Rep J10 NJ60U 3.5 11.875 18 20-0-0 3.5 11.875 J8 NJ60U 1 18-0-0 3.5 11.875 J9 NJ60U 1 12-0-0 J7 NJH 2.5 11.875 40 16-0-0 J6 NJH 2.5 11.875 20 14-0-0 J2 NJH 2.5 11.875 1 6-0-0 2.5 11.875 J1 NJH 2 4-0-0 Rim Board Label Description Pcs Length Width Depth Qty Plies Norbord Rimboard 1.125 11.875 14 Plus 1.125 X 11.875 Hanger Beam/Girder Supported Member Label Pcs Description Skew Slope fasteners fasteners H1 2 HGUS410 46 16d 16 16d 1 HUC410 (Min) H2 14 16d 6 10d H4 13 LT251188 4 10dx1 1/2 2 10dx1 1/2 H6 1 LT351188 4 10dx1 1/2 2 10dx1 1/2 Blocking Label Description Width Depth Qty Plies Pcs Length 2.5 11.875 LinFt BLK1 NJH Varies 39-0-0

- . Framer to verify dimensions on the architectural drawings.
- another member using a face-mounted hanger.
- Install 2x4 blocking @ 24"o/c under parallel non-load bearing walls.
- rimboard/rimjoist.
 5. Refer to Nascor specifier guide for installation works.
- all first level joists which support loading from above exceeding two levels floor or roof.
- fastened as per the hanger manufacturer's standards.

Refer to Multiple Member Connection Detail to ply to ply nailing or

Rim parallel to joists: 1-1/8" rimboard with 2"x4" block (1/16" longer than rim depth @ 16"o/c). All other components and structural elements supporting the floor system such as beams, walls, columns, and foundation walls, and footings including anchorage of components and bracing for lateral stability are the responsibility of Others.

of 5 PSF

The framing shown on this layout may deviate from the architectural and structural drawings. Project Engineer to review and apporve the deviation

ARCHITECTURAL DRAWINGS:

JARDIN DESIGN GROUP INC. 64 Jardin Dr, Suite 3A Date: Rev. 1, 4/26/2018 Project No: 2645

- 2. Nascor CCMC 13535-R
- 3. LVL CCMC -14056-R
- 4. CAN/CSA-O86-09
- 5. CCMC -12787-R APA PR-L310(C)

LSD

2012

40

15

480

360

480

360

360

240

480

240

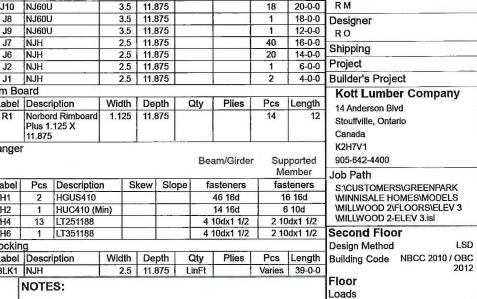
SPF Plywood

Nailed & Glued

Gypsum 1/2"

Version 18.40.162 Powered by iStruct™

This layout is to be used as an installation quide only. It is meant to be used in conjunction with the architectural and structural drawings, not to replace them



2. Double joist only require filler/backer ply when supporting

4. Install single-ply flush window header along inside face of

6. Squash blocks recommended to be installed at end bearing on

. Load transfer blocks to be installed under all point loads.

8. It shall be the frame's responsibility that floor joists and beams are

bolting requirements.

Hatch are represents ceramic tiled floor with an additional dead load

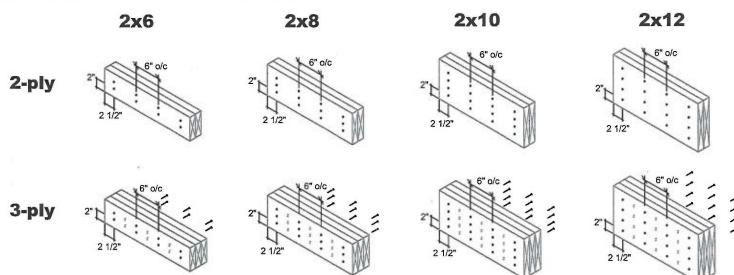
prior to construction.

Model: Millwood 2, Elevation 1

- 1. OBC 2012 O.Reg 332/12 as amended

MULTIPLE MEMBER CONNECTIONS

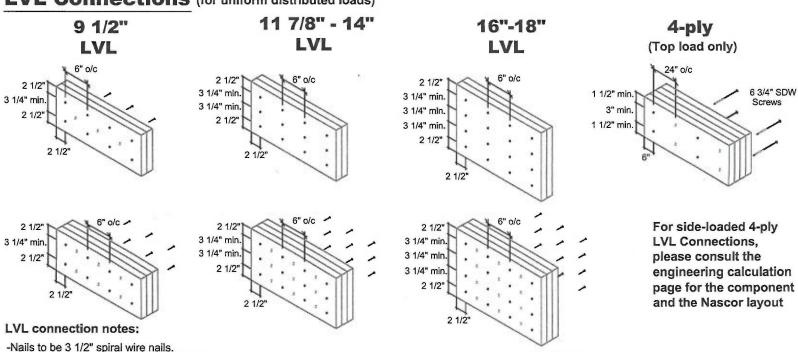
Conventional Connections (for uniform distributed loads)



Conventional connection notes:

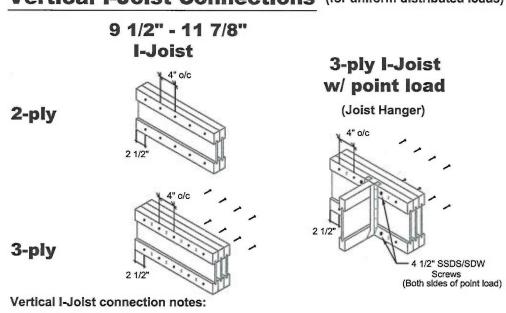
- -Nails to be 3" 10d spiral wire nails.
- -Nails to be located a minimum of 2" from the top and bottom of the member. Start all nails a minimum of 2 1/2" in from ends.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail driven from the opposite side.

LVL Connections (for uniform distributed loads)



- -Nails to be located a minimum of 2 1/2" from the top and bottom of the member. Start all nails a minimum of 2 1/2" in from ends.
- -Minimum 3 1/4" spacing between rows.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail or screw driven from the opposite side.

Vertical I-Joist Connections (for uniform distributed loads)



- -Nails to be 3" spiral wire nails.
- -Nails to be located at centre of top and bottom flanges. Start all nails a minimum of 2 1/2" in from ends.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail driven from the opposite side.

KOTT

KOTT 3228 Moodle Drive Ottawa, ON K2H 7V1 Ph: 613-838-2775

Fx: 613-838-4751

MULTI -PLY CONNECTION DETAILS

Date: November 30, 2016

Scale: NTS

TW0918-058

Engineering Note Page (ENP-2)

REVISION 2009-10-09

Please read all notes prior to installation of the component

DESIGN INFORMATION

This building component is certified as an individual component for the loads and conditions shown on the calculation and drawing page.

The responsibility of the undersigned engineer is <u>only</u> limited to the calculation of this building component for the loads and conditions shown on this drawing.

The responsibility of the undersigned is limited to the verification of the structural capacity of the NASCOR floor joists and LVL beams based on placement as shown on the layout. The loads applied are limited to the gravity effects of the specified loads. The structural integrity of the building and the effect of wind, uplift, seismic, lateral or other forces, calculation of adequate support and anchorage of components, as well as the dimensions and design loads used to calculate components are the responsibility of the overall building designer.

Floor joists and OSB rim board are designed to carry uniformly distributed loads only. Point loads should be transferred through the floor cavity with squash blocks. Structural elements such as walls, posts, connectors, and squash blocks are the responsibility of the overall building designer.

The undersigned engineer disclaims any responsibility for damages as a result of being furnished faulty or incorrect information, specifications and/or designs.

Installation of NASCOR joists is to be carried out in accordance with the current edition of the manufacturer's approved literature available at http://www.nascor.ca.

CODE

This building component is designed in accordance with the National Building Code of Canada, the Ontario Building Code, CCMC and Canadian Standards Association guidelines.

COMPONENT

- 1. The building component used in construction must be the same as indicated on the drawings.
- 2. The building component must be installed and assembled as per specification shown on the drawing and in accordance with the manufacturer's assembly and installation.
- 3. Members consisting of multiple plies must be connected as per the document "Multi-ply Connection Details".
- 4. Pass-thru squash block framing is required at all point loads over bearings.

HANDLING AND INSTALLATION

Do not drill any hole, cut or notch a certified building component without a written preauthorization.



MULTIPLE MEMBER CONNECTIONS

Conventional Connections (for uniform distributed loads)

2x6 2-ply



2x8















Conventional connection notes:

-Nails to be 3" 10d spiral wire nails.

-Nails to be located a minimum of 2" from the top and bottom of the member. Start all nails a minimum of 2 1/2" in from ends.

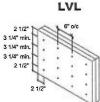
11 7/8" - 14"

- -Number of rows and spacing as per details shown, unless noted otherwise.
 "X" represents nall driven from the opposite side.

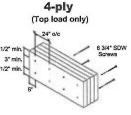
LVL Connections (for uniform distributed loads)

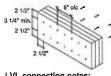
LVL

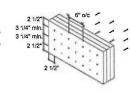


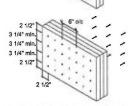


16"-18"









For side-loaded 4-ply LVL Connections, please consult the engineering calculation page for the component and the Nascor layout

LVL connection notes:

- -Nails to be 3 1/2" spiral wire nails.
- -Nalls to be Graze a minimum of 2 1/2" from the top and bottom of the member, Start all nalls a minimum of 2 1/2" in from ends.

 -Minimum 3 1/4" spacing between rows.
- -Number of rows and spacing as per details shown, unless noted otherwise.

 "X" represents nail or screw driven from the opposite side.

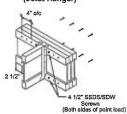
Vertical I-Joist Connections (for uniform distributed loads)

9 1/2" - 11 7/8" I-Joist

2-ply



3-ply I-Joist w/ point load (Joist Hanger)





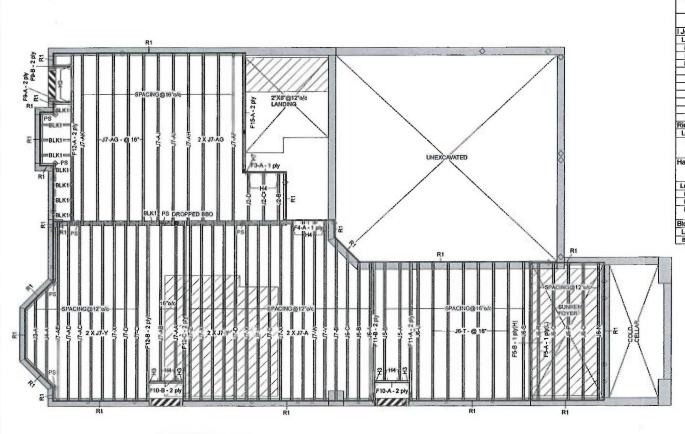
Vertical I-Joist connection notes:

- -Nails to be 3" spiral wire nails.
- -Nalis to be located at centre of top and bottom flanges. Start all nalls a minimum of 2 1/2" in from ends.
 -Number of rows and spacing as per details shown, unless noted otherwise.
 "X" represents nail driven from the opposite side.

KOTT 3228 Moodle Drive Ottawa, ON K2H 7V1 Ph: 613-838-2775

MILLTI PLY CONNECTION

Ground Floor



This certification is to confirm that:

Version 18.40.162 Powered by iStruct's

1. The loads used in the calculation of the attached approved components conform to the floor assembly shown on this layout. 2. The floor joists comply with the Nascor span table for the loads and spacing shown on this layout.

The floor system must be assembled in accordance to the Nascor Specifier Guide. Multi-ply members must be attached together as per the included multiple member connection detail.

All other components and structural elements supporting the floor system such as beams, walls, columns and foundation walls and footings including anchorage of components and bracing for lateral stability are the responsibility of others.



September 13, 2018

This layout is to be used as an installation guide only. It is meant to be used in conjunction with the architectural and structural drawings, not to replace them

1. OBC 2012 O.Reg 332/12 as amende

- 2. Nascor CCMC 13535-R
- 3. LVL CCMC -14058-R
- 4. CAN/CSA-086-09
- 5. CCMC -12787-R APA PR-L310(C)

F15	Forex 2.0E-3	000Fb LVL	1.7	75	11.875		1	2	2	12-0-0	Layout Name MILWOOD 2-ELEV3		
F4	Forex 2.0E-3	000Fb LVL	1.7	75	11.875				1	6-0-0	Design Method LSD		
F3	Forex 2.0E-3	000Fb LVL	1.7	75	11,875				1	4-0-0	Description	_	
loist	(Flush)										Created		
Label	Descr	iption	Wid	h	Depth	C	ity	Plies	Pcs	Length	June 25, 2018		
F12	NJ		1	.5	11.875		3	2	8	16-0-0	Builder	-	
F11	NJ		1	.5	11.875		2	2	4	14-0-0			
F10	NJ		1	.5	11.875		2	2	4	4-0-0	GREENPARK	_	
F9	NJ				11,875		2	2	4	2-0-0	Sales Rep		
J7	NJH		2		11.875				35	18-0-0	RM		
JB	NJH		2		11,875	L.,			10	14-0-0	Designer		
J5	NJH				11.875	_			2	12-0-0	RO		
34	NJH				11.875	_	_		1	10-0-0	Shipping		
13	NJH				11,875		_		1	8-0-0		-	
32	NJH		2	.5	11.875				3	8-0-0	Project	_	
im Bo				-				T		L .	Builder's Project		
	abel Description		Wid		Depth	C	My	Plies	Pcs	Length	Kott Lumber Company		
R1 Norbord Rimboard Plus 1,125 X 11.875		1.12	25	11.875				12	12	14 Anderson Blvd Stoutfville, Ontario			
ange	ſ						Bea	m/Girder		oported ember	Canada K2H7V1 905-642-4400		
leda.	Pcs	Description	n	Sk	sk Sk	pe fasteners		fasteners		Job Path			
H3	6	LT2-151186					41	0dx1 1/2	2 10dx1 1/2		S/CUSTOMERS/GREENPARK		
H4	11	LT251188					4 10dx1 1/2		2 10dx1 1/2		WINNISALE HOMESWODELS		
H7	1	HUCQ1.81/ SDS	19-								WILLWOOD 2/FLOORS/ELEV 3 WILLWOOD 2-ELEV 3.isl		
ockir	nggr										Ground Floor		
abel	Descr	iption	Wid	th	Depth	C	lfy	Plies	Pcs	Length	Design Method LS	D	
BLK1	NJH		2	.5	11.875	П	nFt		Varies	29-0-0	Building Code NBCC 2010 / OB		
	NOTE	S: er to venify di	mensi	ons (on the ar	chite	ctural	drawings			Floor Loads	12	
- 1		le joist only r						upporting			Líve	10	
- 1	anoth	er member u	ising a	face	-mounte	d ha	nger.				Dead	15	
		12x4 blocking								L	Deflection Joist		
- 1		i singre-piy n ard/rimloist	USH WI	noov	y neader	BIOU	å rus	de race di			LL Span L/ 48	30	
- 1	Refer to Nascor specifier guide for installation works. Refer to Nascor specifier guide for installation works. Refer to Nascor specifier guide for installation works.											30	
- 1												30	
- 1		t level joists		supp	ort load	ng fr	om al	ооче ехсе	eding			30	
- 1		wels floor or transfer bloc			eatlant		-11	nt Innda			Deflection Girder		
		liansier bloc							ams are			30	
- 1		and an earth									LL oben D	10	

Width Depth Qty Plies Pcs Length
1.75 11.875 2 14-0-0

fastened as per the hanger manufacturer's standards.

Refer to Multiple Member Connection Detail to ply to ply nailing or boling requirements.

Rim parallel to joists: 1-1/8" rimboard with 2"x4" block (1/16" longer than ind depth of 16"o(c). All other components and structural elements aupporting the floor system such as beams, walls, columns, and foundation walls, and footings including anchrage of components and bracing for fateral stability are the responsibility of Others.

Halch are represents ceramic tiled floor with an additional dead load of 5 PSF

The framing shown on this layout may deviate from the architectural and structural drawings. Project Engineer to review and apporve the deviation prior to construction.

ARCHITECTURAL DRAWINGS:

JARDIN DESIGN GROUP INC. 64 Jardin Dr, Suite 3A Date: Rev. 1, 4/26/2018 Project No: 2645 Model: Milhwood 2, Elevation 1

Legend 0

Ground Floor LVL/LSL (Flush)

Label Description



Point Load Support Load from Above Wall Opening Norbord Rimboard Plus 1.125 X 11.875 NJ 11.875 NJH 11.875 Forex 2.0E-3000Fb LVL 1.76 X 11.875



TL Span L/

LL Cant 2L/

TL Cant 21/ Decking

Deck

Thickness

Fastener

Vibration

240

480

240

3/4

SPF Plywood

Nailed & Glued



Address:

Project:

GREENPARK

9/7/2018 Date:

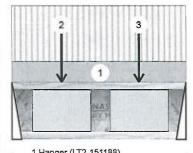
RO Designer:

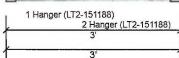
MILLWOOD 2-ELEV 1 Job Name:

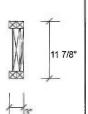
Project #:

2-Ply - PASSED NJ 11.875"

Level: Ground Floor







Wind

0

0

Page 1 of 1

Member Information								
Type:	Girder	Application:	Floor (Residential)					
Plies:	2	Design Method:	LSD					
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012					
Deflection LL:	360	Load Sharing:	No					
Deflection TL:	240	Deck:	Not Checked					
Importance:	Normal	Vibration:	Not Checked					
General Load								
Floor Live:	40 PSF							
Dead:	15 PSF							
		1						

Bearing:	s and	Factored	Reactions	s
Regring	Longt	h Can	Poact D/I	Ih

Live

282

287

Brg

1

2

Unfactored Reactions UNPATTERNED lb (Uplift)

Dead

106

108

Snow

0

0

١	bearing.	aria rac	.coicai	teactions.				
	Bearing	Length	Cap.	React D/L lb	Total	Ld, Case	Ld. Comb.	
	1 - Hanger	2.000"	20%	132 / 423	555	L	1.25D+1.5L	
	2 - Hanger	2.000"	21%	135 / 431	566	L	1.25D+1.5L	

Analysis Results

l	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
ı	Moment	401 ft-lb	1'4 1/2"	9020 ft-lb	0.044 (4%)	1.25D+1.5L	L
l	Unbraced	401 ft-lb	1'4 1/2"	5749 ft-lb	0.070 (7%)	1.25D+1.5L	L
l	Shear	558 lb	2'10 3/4"	3400 lb	0.164 (16%)	1.25D+1.5L	L
	Perm Defl in.	0.001 (L/38142)	1'5 9/16"	0.093 (L/360)	0.010 (1%)	D	Uniform
l	LL Defl inch	0.002 (L/14284)	1'5 1/2"	0.093 (L/360)	0.030 (3%)	L	L
	TL Defl inch	0.003 (L/10392)	1'5 9/16"	0.140 (L/240)	0.020 (2%)	D+L	L

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.

Point

5 Top flange braced at bearings. 6 Bottom flange braced at bearings.											
D	Load Type	Location	Trib Width	Side	Dead	Live	Snow				
1	Tie-In	0-0-0 to 3-0-0	(Span)1-9-8	Тор	15 PSF	40 PSF	0 PSF				
2	Point	0-10-4		Far Face	87 lb	233 lb	0 lb				

WCE OF ONT September 13, 2018

T.L. WISE 100083566

100083566

0 PSF Pass-Thru Framing Squash Block is required aball point loads over bearings

Comments

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

3

NOURS
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to writif the illementations and loadis. Lumber

Dry service conditions, unless noted otherwise
 Upist not to be treated with fire retardant or corro

Handling & Installation

2-2-4

I. Licist flanges must not be cut or drilled
 Refer to latest copy of the Lioist product Information details for framing details, sufflener tables, web hole chart. bridging details, multi-ply fastening details and handlinging-rection details
 Dearge summer to pflange to be laterally restrained by attached shealthing or as specified in engineering notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing lengths—3.5 inches
 For flat roots ponding

86 lb

229 lb

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT **CONTAINS SPECIFICATIONS AND CRITERIA USED** IN THE DESIGN OF THIS COMPONENT.

Manufacturer info Nascor by Kott

Wind

Kott Lumber Company 14 Anderson Blvd, Ontario Canada K2H7V1 905-642-4400



This design

Far Face

isDesign™

Client:

GREENPARK

Project: Address: Date:

9/7/2018

RO Designer:

Job Name: MILLWOOD 2-ELEV 1

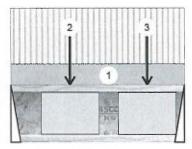
Project #:

Brg

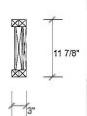
2

2-Ply - PASSED NJ 11.875"

Level: Ground Floor



1 Hanger (LT2-151188) 2 Hanger (LT2-151188)



Wind

0

0

Page 1 of 1

Page 7 of 34

Member Infor	mation		
Type:	Girder	Application:	Floor (Residential)
Plies:	2	Design Method:	LSD
Moisture Condition	n: Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		

E	Bearings and Factored Reactions										
Г	Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.				
	1 - Hanger	2.000"	25%	161 / 514	675	L	1.25D+1.5L				
	2 - Hanger	2.000"	29%	189 / 606	795	L	1.25D+1.5L				

Unfactored Reactions UNPATTERNED Ib (Uplift)

Dead

129

152

Snow

0

0

Live

343

404

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	583 ft-lb	1' 1/4"	9020 ft-lb	0.065 (6%)	1.25D+1.5L	L
Unbraced	583 ft-lb	1' 1/4"	5749 ft-lb	0.101 (10%)	1.25D+1.5L	L
Shear	788 lb	2'10 3/4"	3400 lb	0.232 (23%)	1.25D+1.5L	L
Perm Defl in.	0.001 (L/27610)	1'1 5/16"	0.093 (L/360)	0.010 (1%)	D	Uniform
LL Defl inch	0.003 (L/10370)	1'1 5/16"	0.093 (L/360)	0.030 (3%)	L	L
TL Defl inch	0.004 (L/7538)	1'1 5/16"	0.140 (L/240)	0.030 (3%)	D+L	L

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top flange braced at bearings.
- 6 Bottom flange braced at bearings

DPROFESSIONAL THE
T.L. WISE
100083566
STORY OF OUT PARTY
September 13, 2018

ı	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
l	1	Tie-In	0-0-0 to 3-0-0	(Span)1-9-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
l	2	Point	1-0-4		Far Face	130 lb	346 lb	0 lb	0 lb	J7
	3	Point	2-4-4		Far Face	110 lb	293 lb	0 lb		ruFraming Squash Block is at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads. Lumber

Dry service conditions, unless noted otherwise Uplies not to be treated with fire retardant or corresive

Handling & Installation

- and ling & installation.

 I Joist flanges must not be cut or drilled.

 Refer to latest copy of the I Joist product information defails for framing details, stiffener tables, web hole-teart, bridging details, multi-phy fastening details and handling/erection details.

 Demaged Loists must not be used.

 Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

- Provide lateral support at beering points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 For flat roofs p ponding
 READ ALL NOTES ON T

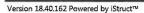
Manufacturer Info Nascor by Kott

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Kott Lumber Company 14 Anderson Blvd, Ontario Canada 905-642-4400



This design







GREENPARK

Project:

Date:

9/7/2018

Page 1 of 1

Designer: RO

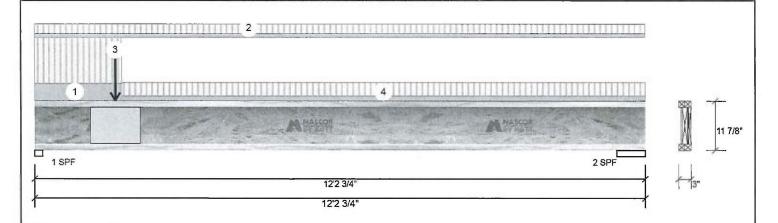
Job Name: MILLWOOD 2-ELEV 1

Project #:

NJ 11.875" 2-Ply - PASSED F11-A

Level: Ground Floor

Unfactored Reactions UNPATTERNED In (Unlift)



Member Imor	mation			Ulliacioi	eu Neaci	IUIS U	MENTIFICIAL	ro in (obiiit	,
Type:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snow	Wind
Plies:	2	Design Method:	LSD	1	512		192	0	0
Moisture Condition	n: Dry	Building Code:	NBCC 2010 / OBC 2012	2	243		91	0	0
Deflection LL:	360	Load Sharing:	No						
Deflection TL:	240	Deck:	Not Checked						
Importance:	Normal	Vibration:	Not Checked						
General Load									
Floor Live:	40 PSF			Bearings	and Fac	tored I	Reactions		
Dead:	15 PSF			Bearing	Length	Cap.	React D/L lb	Total Ld. Ca	se Ld. Comb.
				1-SPF	1.875"	38%	240 / 767	1008 L	1.25D+1.5L
				2-SPF	6.875"	14%	114 / 365	479 L	1.25D+1.5L

Analysis Results

Member Information

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1602 ft-lb	4'6 5/8"	9020 ft-lb	0.178 (18%)	1.25D+1.5L	L
Unbraced	1602 ft-lb	4'6 5/8"	1617 ft-lb	0.991 (99%)	1.25D+1.5L	L
Shear	993 lb	1 1/8"	3400 lb	0.292 (29%)	1.25D+1.5L	L
Perm Defl in.	0.018 (L/7877)	5'6 5/16"	0.388 (L/360)	0.050 (5%)	D	Uniform
LL Defl inch	0.047 (L/2957)	5'6 5/16"	0.388 (L/360)	0.120 (12%)	L	L
TL Defl inch	0.065 (L/2150)	5'6 5/16"	0.581 (L/240)	0.110 (11%)	D+L	L
	•					



- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 5'7" o.c.

5 Bottom flange braced at bearings



September 13, 2018

0 00000111	nango brassa at boarn	90.							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-8-14	(Span)3-3-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 12-2-12	(Span)0-7-12	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	1-7-6		Far Face	108 lb	287 lb	0 lb	0 lb	F10
4	Tie-In	1-8-14 to 12-2-12	(Span)0-11-4	Тор	15 PSF	40 PSF	0 PSF		ru Framing Squash Block is

required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

Celculated Structured Designs is responsible only of the structural adequacy of this component based on the design critients and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads. Lumber

Dry service conditions, unless noted otherwise Usist not to be treated with fire retardant or corrosive

Handling & Installation

1. Ubit finges must not be cut or drilled

2. Refer to latest copy of the IJoist product information details for framing details, sufferer tables, web hole chart, bridging details, multi-ply fastering details and handling/eraction details

3. Demaged Joist must not be used

4. Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

Handling & Installation

Provide lateral support at bearing points to avoid lateral displacement and rotation
 Wab stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 Tot flat ports are:

This design is

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED

IN THE DESIGN OF THIS COMPONENT.

Manufacturer Info

Nascor by Kott

Kott Lumber Company 14 Anderson Blvd, Ontario 905-642-4400



Page 1 of 1

11 7/8

Wind

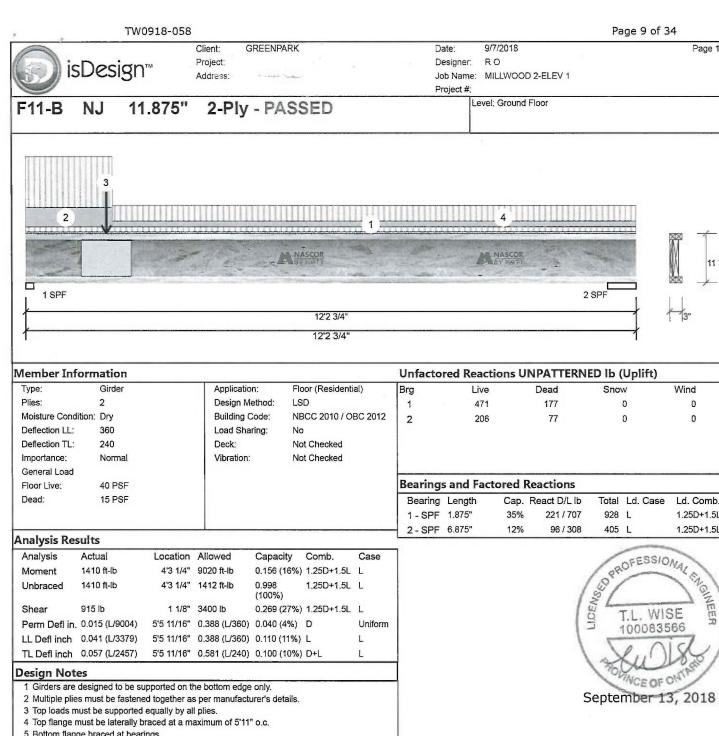
0

0

Ld. Comb.

1.25D+1.5L

1.25D+1.5L



5 Bottom flange braced at bearings

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 12-2-12	(Span)0-3-12	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 1-8-14	(Span)3-3-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	1-7-6		Near Face	106 lb	282 lb	0 lb	0 lb	F10
4	Tie-In	1-8-14 to 12-2-12	(Span) 0-11-12	Тор	15 PSF	40 PSF	0 PSF		ru Framing Squash Block is at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on his design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads. Lumber

Dry service conditions, unless noted otherwise
 Uoist not to be treated with fire retardant or corrosive

Handling & Installation

Intuining a instantation Libial flanges must not be cut or drilled Refer to latest copy of the Lights product information details for framing details, siftener tables, web hole chart, bridging details, multi-ply fastening details and handling/einection details Damaged Libits must not be used

Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 For flat roofs proponding
 READ ALL NOTES ON

This design is

Manufacturer Info Nascor by Kott

READ ALL NOTES ON THIS PAGE AND ON THE CONTAINS SPECIFICATIONS AND CRITERIA USED

ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT IN THE DESIGN OF THIS COMPONENT.

Kott Lumber Company 14 Anderson Blvd, Ontario K2H7V1 905-642-4400



Page 1 of 2



Client:

GREENPARK

Project: Address: Date: 9/7/2018

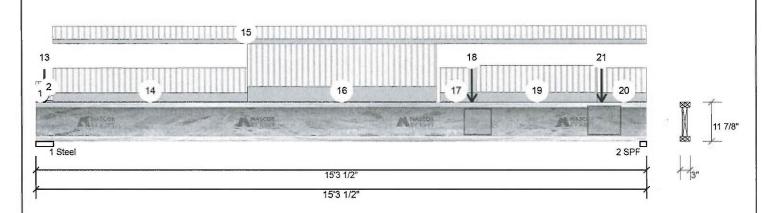
Designer: RO

Job Name: MILLWOOD 2-ELEV 1

Project #:

11.875" 2-Ply - PASSED NJ

Level: Ground Floor



Member Infori	nation			Unfactored Reactions UNPATTERNED Ib (Uplift)							
Type:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind			
Plies:	2	Design Method:	LSD	1	641	269	0	0			
Moisture Condition	: Dry	Building Code:	NBCC 2010 / OBC 2012	2	539	202	0	0			
Deflection LL:	360	Load Sharing:	No								
Deflection TL:	240	Deck:	Not Checked								
Importance:	Normal	Vibration:	Not Checked								
General Load											
Floor Live:	40 PSF			Bearings and	Factored	Reactions					
Dead:	15 PSF			Bearing Leng	th Cap.	React D/L lb	Total Ld. Case	Ld. Comb.			
				1 - Steel 5.250	" 38%	337 / 961	1298 L	1.25D+1.5L			
		1		2-SPF 1.875	" 40%	253 / 809	1061 L	1.25D+1.5L			

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	3724 ft-lb	8' 3/16"	9020 ft-lb	0.413 (41%)	1.25D+1.5L	L
Unbraced	3724 ft-lb	8' 3/16"	3737 ft-lb	0.997 (100%)	1.25D+1.5L	L
Shear	1052 lb	15'2 3/8"	3400 lb	0.309 (31%)	1.25D+1.5L	L)
Perm Defl in.	0.062 (L/2891)	7'10 3/8"	0.494 (L/360)	0.120 (12%)	D	Uniform
LL Defl inch	0.164 (L/1083)	7'10 3/8"	0.494 (L/360)	0.330 (33%)	L	L
TL Defl inch	0.226 (L/788)	7'10 3/8"	0.741 (L/240)	0.300 (30%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 3'9" o.c.

5 Bottom flange braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 0-5-4	(Span)0-3-12	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 0-5-4	(Span)0-8-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	0-2-10		Тор	1 lb	3 lb	0 lb	0 lb	J7
4	Point	0-2-10		Тор	1 lb	4 lb	0 lb	0 lb	J7
5	Point	0-2-10		Тор	1 lb	3 lb	0 lb	0 lb	J7
6	Point	0-2-10		Тор	1 lb	0 lb	0 lb	0 lb	Wall Self Weight
7	Point	0-2-10		Тор	21 lb	56 lb	0 lb	0 lb	J7
CONTROL CONTROL	CONTRACT LAND CONTRACT CONTRAC								

Continued on page 2...

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads. Lumber

Dry service conditions, unless noted otherwise
 Upoist not to be treated with fire retardant or corrosive

Handling & Installation

- andling & Installation.

 I Joist flanges must not be cut or drilled.
 Refer to latest copy of the Libist product information details for framing details, suffener tables, web hole teach. bridging details, multi-ply fastening details and handling/erection details.

 Demaged Lolists must not be used.
 Design assumes top flange to be laterally restrained by attached shealthing or as specified in engineering notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 For flat roofs provide READ ALL NOTES Of ponding

7.

This design is v

READ ALL NOTES ON THIS PAGE AND ON THE IN THE DESIGN OF THIS COMPONENT.

ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT **CONTAINS SPECIFICATIONS AND CRITERIA USED**

Manufacturer Info

Nascor by Kott



September 13, 2018

Kott Lumber Company 14 Anderson Blvd, Ontario Canada K2H7V1 905-642-4400





Page 2 of 2



Client:

Project: Address: GREENPARK

9/7/2018 Date

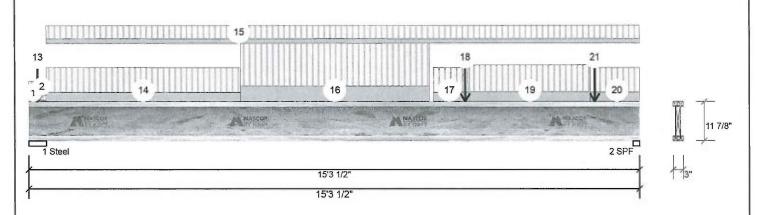
RO Designer:

Job Name: MILLWOOD 2-ELEV 1

Project #:

2-Ply - PASSED 11.875"

Level: Ground Floor



Continued	from page 1								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
8	Point	0-2-10		Тор	22 lb	59 lb	0 lb	0 lb	J7
9	Point	0-2-10		Тор	2 lb	5 lb	0 lb	0 lb	J7
10	Point	0-2-10		Тор	20 lb	0 [b	0 lb	0 lb	Wall Self Weight
11	Point	0-2-10		Тор	9 lb	25 lb	0 lb	0 lb	J7
12	Point	0-2-10		Тор	10 lb	26 lb	0 lb	0 lb	J7
13	Point	0-2-10		Тор	9 lb	0 lb	0 lb	0 lb	Wall Self Weight
14	Tie-In	0-5-4 to 5-3-10	(Span)1-7-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
15	Tie-In	0-5-4 to 15-3-8	(Span) 0-10-12	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
16	Tie-In	5-3-10 to 10-0-8	(Span)2-9-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
17	Tie-In	10-1-10 to 11-0-10	(Span)1-7-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
18	Point	10-11-2		Far Face	18 lb	49 lb	0 lb	0 lb	F9
19	Tie-In	11-0-10 to 14-0-10	(Span)1-9-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
20	Tie-In	14-0-10 to 15-3-8	(Span)1-7-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
21	Point	14-2-2		Far Face	25 lb	66 lb	0 lb	0 lb	F9

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

Colculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

Handling & Installation

And ling & Installation.

Joist flanges must not be cut or drilled.
Refer to latest copy of the Liotst product information details for framing details, sufferer tables, web note chart, bridging details, multiply fastening details and handling/erection details.

Demaged Lolots must not be used.
Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

Provide lateral support at beering points to avoid lateral displacement and rotation
 Web stiffeness for point load as shown Minimum point load bearing length>= 3.5 inches
 For flat roofs provide proper drainage to prevent ponding

This design is valid until 7/10/2021

Manufacturer Info Nascor by Kott

Kott Lumber Company 14 Anderson Blvd, Ontario Canada 905-642-4400



Page 1 of 1



Client:

Project: Address: GREENPARK

Date: Designer:

9/7/2018

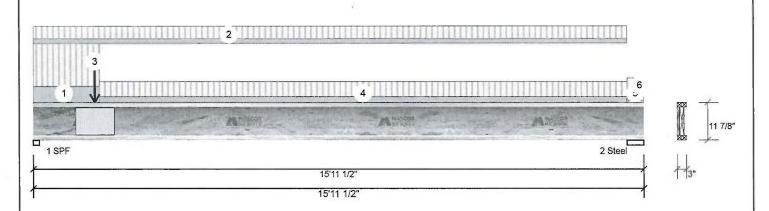
RO

Job Name: MILLWOOD 2-ELEV 1

Project #:

2-Ply - PASSED 11.875" F12-B NJ

Level: Ground Floor



Member Inform	nation			Unfactored Reactions UNPATTERNED Ib (Uplift)							
Type:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snov	N	Wind	
Plies:	2	Design Method:	LSD	1	712		267		0	0	
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012	2	376		141		0	0	
Deflection LL:	360	Load Sharing:	No	-							
Deflection TL:	240	Deck:	Not Checked								
Importance:	Normal	Vibration:	Not Checked								
General Load		100000000000000000000000000000000000000									
Floor Live:	40 PSF			Bearings	and Fac	tored F	Reactions				
Dead:	15 PSF			Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.	
				1-SPF	1.875"	53%	334 / 1068	1402	L	1.25D+1.5L	
				2 - Steel	5.250"	22%	176 / 563	739	L	1.25D+1.5L	

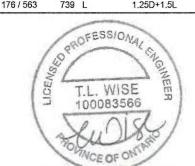
Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	3112 ft-lb	6'11 1/2"	9020 ft-lb	0.345 (34%)	1.25D+1.5L	L
Unbraced	3112 ft-lb	6'11 1/2"	3135 ft-lb	0.993 (99%)	1,25D+1.5L	L
Shear	1386 lb	1 1/8"	3400 lb	0.408 (41%)	1.25D+1.5L	L
Perm Defl in.	0.057 (L/3233)	7'6 13/16"	0.516 (L/360)	0.110 (11%)	D	Uniform
LL Defl inch	0.153 (L/1213)	7'6 13/16"	0.516 (L/360)	0.300 (30%)	L	L
TL Defl inch	0.211 (L/882)	7'6 13/16"	0.774 (L/240)	0.270 (27%)	D+L	L



- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 4'2" o.c.

5 Bottom flange braced at bearings.



September 13, 2018

3 DOMOIT	nange braced at bearin	ys.						
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind Comments
1	Tie-In	0-0-0 to 1-8-14	(Span)3-3-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF
2	Tie-In	0-0-0 to 15-6-4	(Span) 0-11-12 to 0-11-12	Тор	15 PSF	40 PSF	0 PSF	0 PSF
3	Point	1-7-6		Near Face	129 lb	343 lb	0 lb	Olb F10
4	Tie-In	1-8-14 to 15-6-4	(Span)1-1-12 to 1-1-12	Тор	15 PSF	40 PSF	0 PSF	Pass-Thru Framing Squash Block is required at all point loads over bearings
5	Tie-In	15-6-4 to 15-11-8	(Span)0-5-4	Тор	15 PSF	40 PSF	0 PSF	Refer to Multiple Member Connection
6	Tie-In	15-6-4 to 15-11-8	(Span) 0-10-12	Тор	15 PSF	40 PSF	0 PSF	Details for ply to ply nailing or bolting requirements

Notes

Calculated Structured Designs is responsible only of the structural adequacy of fils component based on the design criteria and leadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the Intended application, and to verify the dimensions and loads. Lumber

Dry service conditions, unless noted otherwise
 Uplish not to be treated with fire retardant or corrosive

Handling & Installation

- AIRCHING & INSTALLATION

 Librist flanges must not be cut or drilled

 Refer to latest copy of the Librist product information details for framing details, suffener tables, web hole chart. bridging details, multi-ply fastening details and handling/erection details

 Damaged Librist must not be used

 Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.
- Provide lateral support at bearing points to avoid lateral displacement and rotation

lateral displacement and rotation

6. Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches

7. For flat roofs proponding

READ ALL NOTES ON

This design is

READ ALL NOTES ON THIS PAGE AND ON THE

ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Manufacturer Info

Nascor by Kott

Kott Lumber Company 14 Anderson Blvd, Ontario Canada K2H7V1 905-642-4400





Page 1 of 2



Client:

Project:

GREENPARK

Address:

Date: 9/7/2018

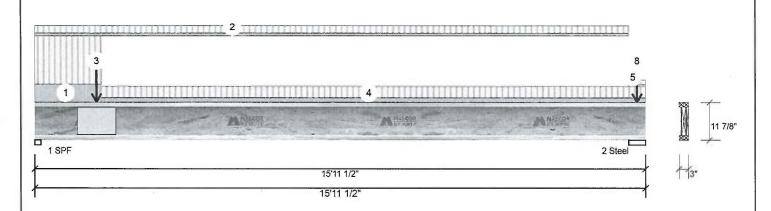
Designer:

Job Name: MILLWOOD 2-ELEV 1

Project #:

2-Ply - PASSED NJ 11.875"

Level: Ground Floor



Member Infor	rmation		Unfactored Reactions UNPATTERNED Ib (Uplift)							
Type:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snov	v	Wind
Plies:	2	Design Method:	LSD	1	649		244	()	0
Moisture Conditio	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	417		179)	0
Deflection LL:	360	Load Sharing:	No	1 -						
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal	Vibration:	Not Checked	1						
General Load		100000000000000000000000000000000000000								
Floor Live:	40 PSF			Bearings a	nd Fact	ored R	Reactions			
Dead:	15 PSF			Bearing Le	ength	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
				1-SPF 1.	.875"	48%	305 / 973	1278	L	1.25D+1.5L
				2 - Steel 5.	250"	25%	223 / 625	849	1	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2271 ft-lb	6'1 5/8"	9020 ft-lb	0.252 (25%)	1.25D+1.5L	L
Unbraced	2271 ft-lb	6'1 5/8"	2271 ft-lb	1.000 (100%)	1.25D+1.5L	L
Shear	1264 lb	1 1/8"	3400 lb	0.372 (37%)	1.25D+1.5L	L
Perm Defl in.	0.042 (L/4432)	7'4 5/8"	0.516 (L/360)	0.080 (8%)	D	Uniform
LL Defl inch	0.112 (L/1664)	7'4 3/4"	0.516 (L/360)	0.220 (22%)	L	L
TL Defl inch	0.154 (L/1210)	7'4 3/4"	0.774 (L/240)	0.200 (20%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 4'10" o.c.

5 Bottom flange braced at bearings.

		0							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-8-14	(Span)3-3-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 15-6-4	(Span)0-6-4 to 0-6-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	1-7-6		Far Face	152 lb	404 lb	0 lb	0 lb	F10
4	Tie-In	1-8-14 to 15-11-8	(Span)0-9-4 to 0-9-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
5	Tie-In	15-6-4 to 15-11-8	(Span)0-3-12	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
6	Point	15-8-14		Тор	32 lb	85 lb	0 lb	0 lb	J7

Continued on page 2...

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the Intended application, and to verify the dimensions and loads. Lumber

Dry service conditions, unless noted otherwise Lioist not to be treated with fire retardant or corrosive

Handling & Installation

- 1.
- Andling & installation.

 Lioist flanges must not be cut or drilled.
 Refer to latest copy of the Lioist product information details for framing details, suffience tables, web note thank. bridging details, multi-ply fastening details and handling/erection details.

 Demaged Lolists must not be used.
 Design assumes top flenge to be laterally restrained by attached sheathing or as specified in engineering notes.

Provide lateral support at bearing points to avoid lateral displacement and rotation.
 Web stiffeness for point load as shown Minimum point load bearing length>= 3.5 inches
 READ ALL NOTES ON

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Manufacturer Info Nascor by Kott

Kott Lumber Company 14 Anderson Blvd, Ontario Canada K2H7V1 905-642-4400

T.L. WISE 100083566

100083566

VCE OF ONT

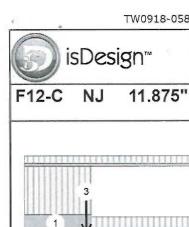
September 13, 2018



This design is



Page 2 of 2



Client: GREENPARK Project:

Address:

Date: Designer:

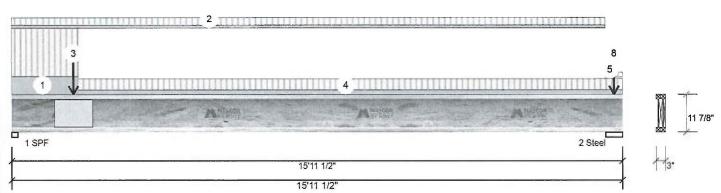
9/7/2018

RO Job Name: - M'LLWOOD 2-ELEV 1

Project #:

2-Ply - PASSED

Level: Ground Floor



.Continued from page 1

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
7	Point	15-8-14		Тор	30 lb	80 lb	0 lb	0 lb	J7
8	Point	15-8-14		Тор	22 lb	0 lb	0 lb	0 lb	Wall Self Weight

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer endor the contractor to ensure the component suitability of the Intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 Uplist not to be treated with fire retardant or corrosive

Handling & Installation

Handling & Installation

1. Joist flanges must not be cut or drilled

2. Refer to latest copy of the Jloist product information details for framing details, suffering retails, web hole chart, bridging details, multi-liy fastening details and handling/erection details

3. Damaged looists must not be used

4. Design assumes top flange to be laterally restrained by attached shealthing or as specified in engineering notes.

Provide lateral support at bearing points to avoid lateral displacement and rotation.
 Web stiffeness for point load as shown Minimum point load bearing length>= 3.5 Inches
 For flat roofs provide proper drainage to prevent ponding.

Manufacturer Info Nascor by Kott

Kott Lumber Company 14 Anderson Blvd, Ontario Canada K2H7V1 905-642-4400

This design is valid until 7/10/2021



Project: Address: GREENPARK

Date: 9/10/2018 Designer:

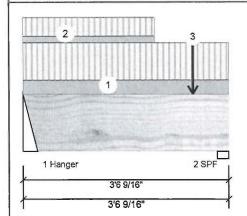
Job Name: MILLWOOD 2-ELEV 1

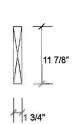
Project #:

Forex 2.0E-3000Fb LVL F3-A

1,750" X 11,875" - PASSED

Level: Ground Floor





Page 1 of 1

Member Inf	ormation						Unfacto	red React	ions U	NPATTERN	ED lb	(Uplift)	
Type: Plies: Moisture Cond Deflection LL: Deflection TL: Importance: General Load Floor Live:	Girder 1 1ition: Dry 360 240 Normal		Application Design M Building C Load Sha Deck: Vibration:	ethod: LS Code: NE ring: No No	3CC 2010 / O		Brg 1 2 Bearing	Live 555 510	tored	Dead 218 201 Reactions	Sno	w 0 0	Wind 0 0
Dead:	15 PSF						Bearing 1 - Hanger			React D/L lb	Total 1106		Ld. Comb. 1.25D+1.5L
Analysis Re	sults						2-SPF	2.375"	40%	251 / 765	1016	L	1.25D+1.5L
Analysis Moment Unbraced Shear Perm Defi in. LL Defl inch TL Defl inch Design Not 1 Fill all hang	(L/26669) 0.004 (L/10472) 0.005 (L/7520)	1'9 3/8"	Allowed 17130 ft-lb 13259 ft-lb 5798 lb 0.108 (L/360) 0.108 (L/360) 0.161 (L/240)	0.129 (13%) 0.010 (1%) 0.030 (3%)	L	L				HOR	O PAR	L. WIS	80/
2 Girders are 3 Top braced	designed to be su	pported on ti	he bottom edge	only.						S	epter	nber 13	A

Doctor	bradda at boarings.								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Part, Uniform	0-0-0 to 3-6-9		Тор	79 PLF	210 PLF	0 PLF	0 PLF	
2	Part. Uniform	0-0-0 to 2-3-3		Near Face	39 PLF	103 PLF	0 PLF	0 PLF	
3	Point	2-11-3		Near Face	33 lb	87 lb	0 lb	0 lb	J2
	Self Weight				5 PLF				ru Framing Squash Block is I at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

Celculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and leadings shown. It is the responsibility of the customer endor the contractor to ensure the component suitability of the Intended application, and to verify the dimensions and loads. Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosis

Handling & Installation

Handling & Installation

1. IVJ beams must not be cut or drilled

2. Refer to menufacturer's product information regarding installation requirements. multi-ply fastering details, beam strength values, and code approvals

3. Damaged Beams must not be used

4. Design assumes top edge is laterally restrained

5. Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

This design is v

Manufacturer Info Forex APA: PR-L318

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Kott Lumber Company 14 Anderson Blvd, Ontario Canada K2H7V1 905-642-4400





Client: GREENPARK

Project: Address: Date:

9/7/2018 Designer: RO

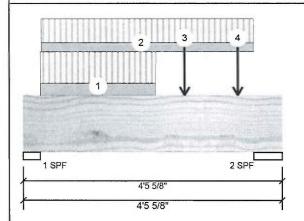
Job Name: MILLWOOD 2-ELEV 1

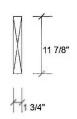
Project #:

Forex 2.0E-3000Fb LVL

1.750" X 11.875" - PASSED

Level: Ground Floor





Page 1 of 1

Member Infor	mation			Unfactor	red React	ions U	NPATTERNI	ED lb (Uplift)	
Type:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snov	v	Wind
Plies:	1	Design Method:	LSD	1	1024		395		0	0
Moisture Conditio	n: Dry	Building Code:	NBCC 2010 / OBC 2012	2	1099		424	1	0	0
Deflection LL:	360	Load Sharing:	No	-						
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal	Vibration:	Not Checked	1						
General Load										
Floor Live:	40 PSF			Bearings	and Fac	tored l	Reactions			
Dead:	15 PSF			Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
				1-SPF	3.500"	54%	493 / 1536	2029	L	1.25D+1.5L
				2-SPF	5.875"	34%	529 / 1649	2178	L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2002 ft-lb	2'1 9/16"	17130 ft-lb	0.117 (12%)	1.25D+1.5L	L
Unbraced	2002 ft-lb	2'1 9/16"	11720 ft-lb	0.171 (17%)	1.25D+1.5L	L
Shear	2314 lb	3' 5/8"	5798 lb	0.399 (40%)	1.25D+1.5L	L
Perm Defl in.	0.004 (L/10769)	2'1 11/16"	0.127 (L/360)	0.030 (3%)	D	Uniform
LL Defl inch	0.011 (L/4138)	2'1 11/16"	0.127 (L/360)	0.090 (9%)	L	L
TL Defl inch	0.015 (L/2989)	2'1 11/16"	0.191 (L/240)	0.080 (8%)	D+L	L



September 13, 2018

Design N	otes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Top braced at bearings.
- 3 Bottom braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Part. Uniform	0-3-8 to 2-3-8		Near Face	120 PLF	319 PLF	0 PLF	0 PLF	
2	Part. Uniform	0-3-12 to 3-11-12		Тор	90 PLF	240 PLF	0 PLF	0 PLF	
3	Point	2-9-8		Near Face	115 lb	305 lb	0 lb	0 lb	J7
4	Point	3-8-8		Near Face	112 lb	300 lb	0 lb	0 lb	J7
le le	Self Weight				5 PLF				ru Framing

Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

NOTES:
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to wrift the dimensions and loads. Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or conditions.

Handling & Installation

- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- approvals

 Damaged Beams must not be used

 Design assumes top edge is laterally restrained

 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info APA: PR-L318

IN THE DESIGN OF THIS COMPONENT.

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED

Kott Lumber Company 14 Anderson Blvd, Ontario Canada K2H7V1 905-642-4400



This design is



Page 1 of 1



Client:

Project:

GREENPARK

Address:

Date: 9/7/2018

Designer: RO

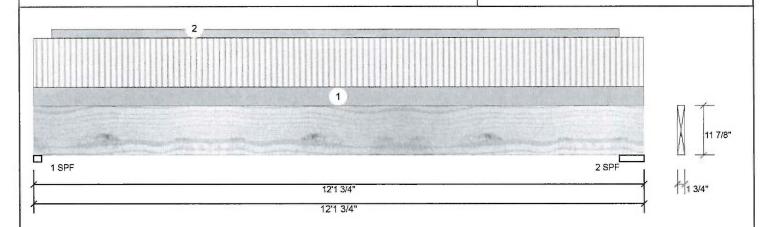
Job Name: MILLWOOD 2-ELEV 1

Project #:

Forex 2.0E-3000Fb LVL

1.750" X 11.875" - PASSED

Level: Ground Floor



Member Infor	mation			Unfactore	ed React	ions U	NPATTERNI	ED Ib (l	Uplift)	
Type:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snow	1	Wind
Plies:	1	Design Method:	LSD	1	73		67	0)	0
Moisture Condition	n: Dry	Building Code:	NBCC 2010 / OBC 2012	2	77		70	0)	0
Deflection LL:	360	Load Sharing:	No							
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal	Vibration:	Not Checked							
General Load										
Floor Live:	40 PSF			Bearings	and Fact	tored F	Reactions			
Dead:	15 PSF			Bearing L	_ength	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
				1-SPF 1	1.875"	10%	83 / 109	192	L	1.25D+1.5L
				2-SPF 5	5.875"	3%	88 / 115	203	L	1.25D+1.5L

Analysis Results

Γ	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
l	Moment	553 ft-lb	5'10 7/8"	17130 ft-lb	0.032 (3%)	1.25D+1.5L	L
l	Unbraced	553 ft-lb	5'10 7/8"	3868 ft-lb	0.143 (14%)	1.25D+1.5L	L
ŀ	Shear	158 lb	1'1"	5798 lb	0.027 (3%)	1.25D+1.5L	L
l	Perm Defl in.	0.011 (L/13107)	5'10 7/8"	0.388 (L/360)	0.030 (3%)	D	Uniform
l	LL Defl inch	0.012 (L/12101)	5'10 7/8"	0.388 (L/360)	0.030 (3%)	L	L
	TL Defl inch	0.022 (L/6292)	5'10 7/8"	0.581 (L/240)	0.040 (4%)	D+L	L

T.L. WISE 100083566 100083566 WCE OF ON September 13, 2018

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 3 Bottom braced at bearings.

2 Top braced at bearings.	

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 12-1-12	(Span)0-7-6	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Part. Uniform	0-4-6 to 11-7-15		Тор	2 PLF	0 PLF	0 PLF	0 PLF	
	Self Weight				5 PLF				

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads. Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

and Ilmg & Installation
LVL beams must not be cut or drilled
Refer to manufacturer's product information
regarding installation requirements, multi-pil
festening details, beam strength values, and code
approvals
Damaged Beams must not be used
Destign assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

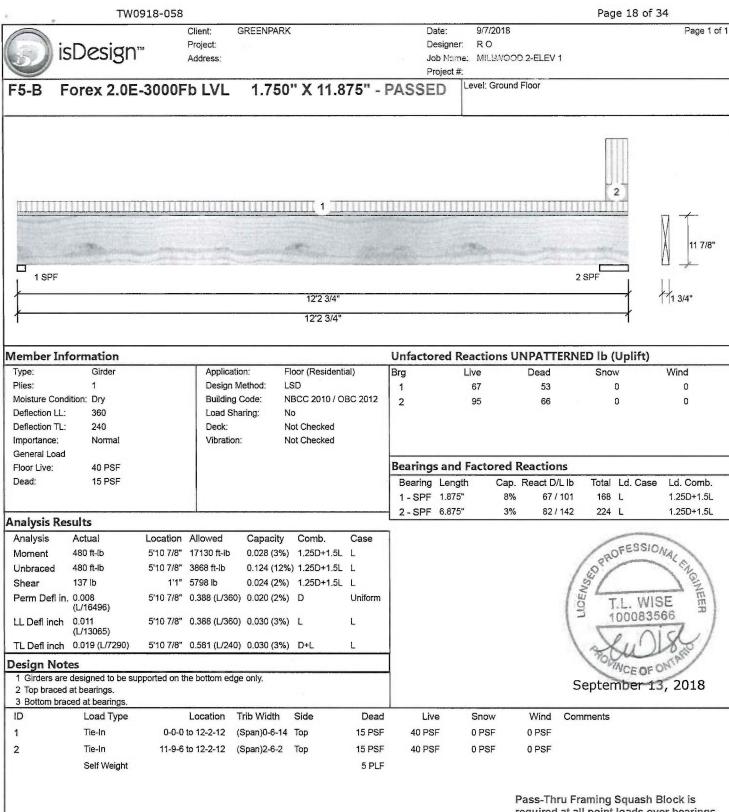
Manufacturer Info Forex APA: PR-L318

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT **CONTAINS SPECIFICATIONS AND CRITERIA USED** IN THE DESIGN OF THIS COMPONENT.

Kott Lumber Company 14 Anderson Blvd, Ontario Canada K2H7V1 905-642-4400



This design is



required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design critieria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the Intended application, and to verify the dimensions and loads. Lumber

Handling & Installation

LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code

approvals
Damaged Beams must not be used
Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

This design is

Manufacturer Info APA: PR-L318

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT **CONTAINS SPECIFICATIONS AND CRITERIA USED** IN THE DESIGN OF THIS COMPONENT.

Kott Lumber Company 14 Anderson Blvd, Ontario Canada K2H7V1





Project: Address: Date:

Brg

1

Bearing Length

1 - SPF 1.875" 2.000"

2 -Hanger 9/7/2018

RO

MILLWOOD 2-ELEV 1

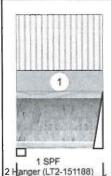
Job Name: Project #:

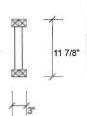
Designer:

NJ 11.875" 2-Ply - PASSED

GREENPARK

Level: Ground Floor





Wind

Page 1 of 1

1	1'5 7/8"	· ·
1	1'5 7/8"	-

Type:	Girder	Application:	Floor (Residential)
Plies:	2	Design Method:	LSD
Moisture Conditi	on: Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		

2	49	18	0	0
Bearings	and Factored	Reactions		

Dead

Unfactored Reactions UNPATTERNED lb (Uplift)

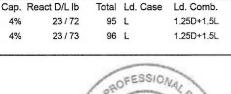
4%

4%

Live

48

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	27 ft-lb	8 7/8"	9020 ft-lb	0.003 (0%)	1.25D+1.5L	L
Unbraced	27 ft-lb	8 7/8"	8539 ft-lb	0.003 (0%)	1.25D+1.5L	L
Shear	83 lb	1 1/8"	3400 lb	0.024 (2%)	1.25D+1.5L	L
Perm Defl in.	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
LL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
TL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		





1 Fill all hanger nailing holes.

Design Notes

- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top flange braced at bearings.
- 6 Bottom flange braced at bearings

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-5-14	(Span)3-3-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the Intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 Idolst not to be treated with fire retardant or corrosive

Handling & Installation

- andling & Installation

 Noist flanges must not be cut or drilled

 Rafer to latest copy of the Lloist product information
 details for framing details, sutifiener tables, web hole
 chart, bridging details, multi-py fastening details and
 handling/erection details

 Damaged Lloists must not be used

 Design assumes top flange to be laterally restrained
 by attached sheathing or as specified in engineering
 notes.

This design is

Provide lateral support at bearing points to avoid lateral displacement and rotation.
 Web stiffeness for point load as shown Minimum point load bearing length >= 3.5 inches.
 For flat roofs provided PREAD ALL NOTES Of ponding
 MINIMEDIALS NOTE

Manufacturer Info Nascor by Kott

READ ALL NOTES ON THIS PAGE AND ON THE

ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Kott Lumber Company 14 Anderson Blvd, Ontario Canada K2H7V1 905-642-4400



Version 18.40,162 Powered by iStruct™





Client: **GREENPARK**

Project: Address: Date:

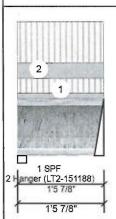
9/7/2018

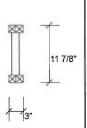
RO Designer: MILLWOOD 2-ELEV 1 Job Name:

Project #:

2-Ply - PASSED NJ 11.875"

Level: Ground Floor





Page 1 of 1

lember Info	rmation			Unfactor	red React	ions U	NPATTERNI	ED lb (Uplift)	
Туре:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snow	Wind
Plies:	2	Design Method:	LSD	1	65		24	0	0
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	66		25	0	0
Deflection LL:	360	Load Sharing:	No	1 -					
Deflection TL:	240	Deck:	Not Checked						
Importance:	Normal	Vibration:	Not Checked						
General Load		1 p							
Floor Live:	40 PSF			Bearings	and Fac	tored F	Reactions		
Dead:	15 PSF			Bearing	Length	Сар.	React D/L lb	Total Ld. Case	Ld. Comb.
				1-SPF	1.875"	5%	31 / 98	128 L	1.25D+1.5L
				2-	2 000"	5%	31 / 99	130 L	1.25D+1.5I

Hanger

Δna	veis	Resu	te
MILLIA	Lysis	17C3C	13

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	36 ft-lb	8 7/8"	9020 ft-lb	0.004 (0%)	1.25D+1.5L	L
Unbraced	36 ft-lb	8 7/8"	8539 ft-lb	0.004 (0%)	1.25D+1.5L	L
Shear	112 lb	1 1/8"	3400 lb	0.033 (3%)	1.25D+1.5L	L
Perm Defl in	. 0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
LL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
TL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top flange braced at bearings
- 6 Bottom flange braced at bearings

	OPROFESSIONAL ON
(No.	
LIGHT	T.L. WISE 100083566
/	NO WINCE OF ON THE

September 13, 2018

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-5-14	(Span)1-1-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 1-5-14	(Span)3-3-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the Intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 Usist not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- andling & Installation.

 Lioist flanges must not be cut or drilled.
 Refer to latest copy of the Lioist product information details for framing details, stiffener tables, web note chart, bridging details, multi-ply fastering details and handling/erection details.

 Damaged Loists must not be used.
 Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

5. Provide lateral support at bearing points to avoid lateral displacement and rotation.

6. Web stiffeners for point load as shown Minimum point load bearing length 2-3.5 inches.

7. For fat roofs provided the provided in the provided in

This design is

READ ALL NOTES ON THIS PAGE AND ON THE IS AN INTEGRAL PART OF THIS DRAWING AS IT

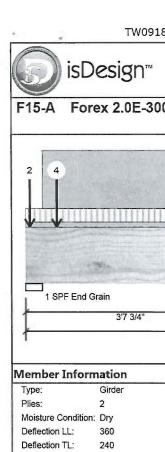
ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Manufacturer Info

Nascor by Kott

Kott Lumber Company 14 Anderson Blvd, Ontario K2H7V1 905-642-4400





GREENPARK

Client: Project:

Address:

Date:

9/10/2018

Page 1 of 2

Designer: RO

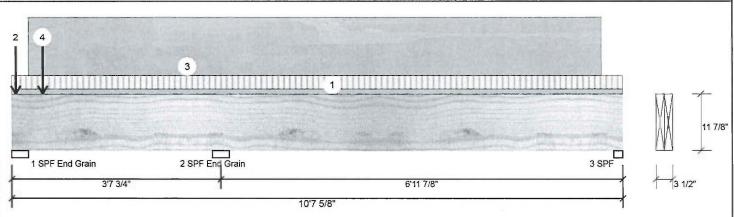
Job Name: MILLWOOD 2-ELEV 1

Project #:

Forex 2.0E-3000Fb LVL

1.750" X 11.875"

2-Ply - PASSED Level: Ground Floor



Brg

1

2

3

Туре:	Girder	Application:	Floor (Residential)
Plies:	2	Design Method:	LSD
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF	_	

Bearing	Bearings and Factored Reactions											
	Length		React D/L lb	Total	Ld. Case	Ld. Comb.						
1 - SPF End Grain	3.500"	24%	578 / 1504	2082	L_	1.25D+1.5L						
2 - SPF End Grain	3.500"	17%	995 / 0	995	Uniform	1.4D						
3-SPF	1.875"	13%	348 / 0	348	Uniform	1.4D						

Unfactored Reactions UNPATTERNED lb (Uplift)

Dead

484

679

259

Snow

0

0

0

Wind

0

0

Live

986

176

52

Analysis Results Analysis Actual Location Allowed Comb. Case Capacity Neg Moment -617 ft-lb 3'7 3/4" 22269 ft-lb 0.028 (3%) Uniform 1.4D -617 ff-lb 3'7 3/4" 22269 ft-lb 1.4D 0.028 (3%) Uniform Unbraced Pos Moment 521 ft-lb 7'9" 22269 ft-lb 0.023 (2%) Uniform Unbraced 521 ft-lb 7'9" 21873 ft-lb 0.024 (2%) Uniform Shear 421 lb 4'7 5/8" 7537 lb 0.056 (6%) 1.4D Uniform Perm Defl in. 0.005 7'3 7/16" 0.230 (L/360) 0.020 (2%) D Uniform (L/17692) LL Defl inch 0.000 (L/999) 0 999.000 (L/0) 0.000 (0%) TL Defl inch 0.006 7'3 3/8" 0.345 (L/240) 0.020 (2%) D+L _L (L/14724)

Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on full section width.



September 13, 2018

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the Intended application, and to verify the dishability of the Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or col

chemicals

Handling & Installation

Anduling & InStallation

LVL beams must not be cut or drilled

Refer to manufacturer's product information
regarding installation requirements, multi-ply
fastaning details, beam strength values, and code
approvals

Damaged Beams must not be used

Design assumes top edge is laterally restrained

Provide lateral support at bearing points to avoid
lateral displacement and rotation

Manufacturer Info

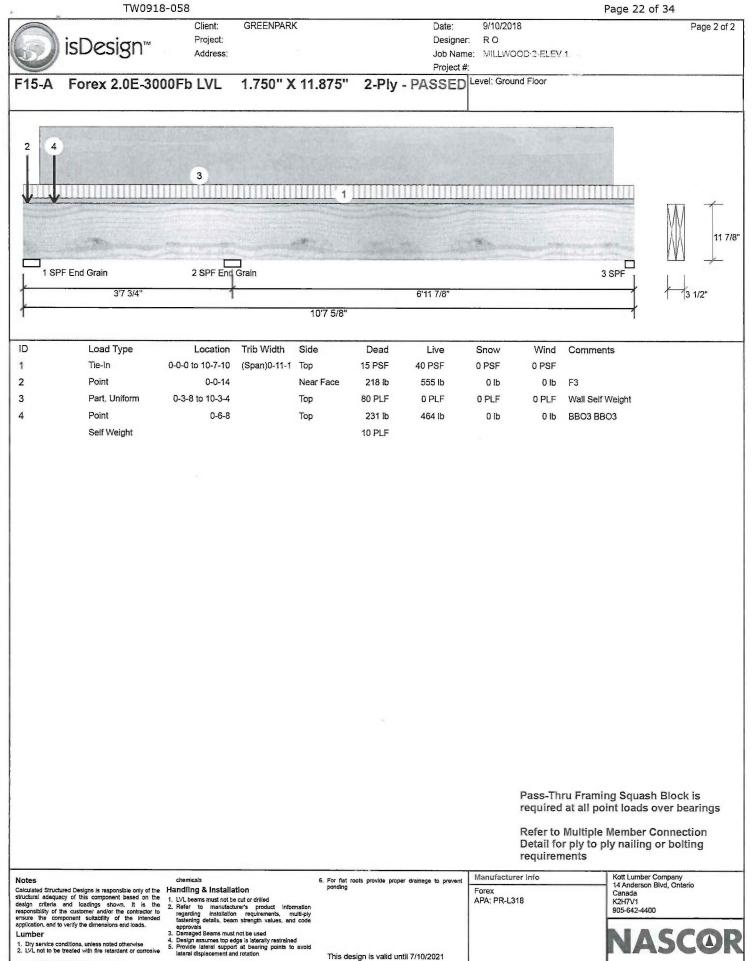
Forex APA: PR-L318

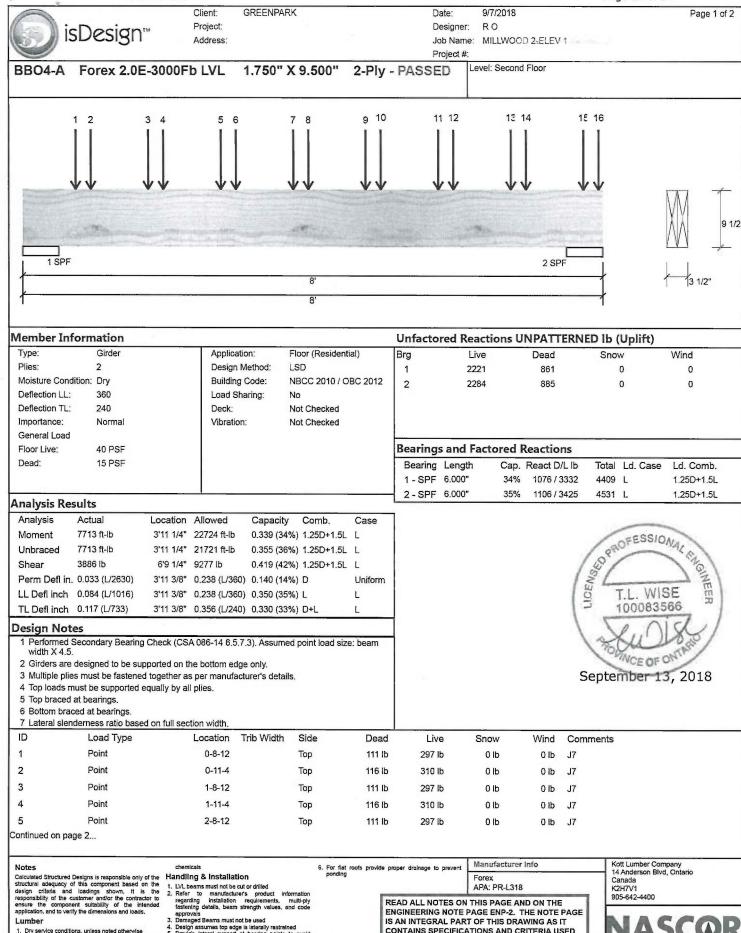
READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Kott Lumber Company 14 Anderson Blvd, Ontario K2H7V1





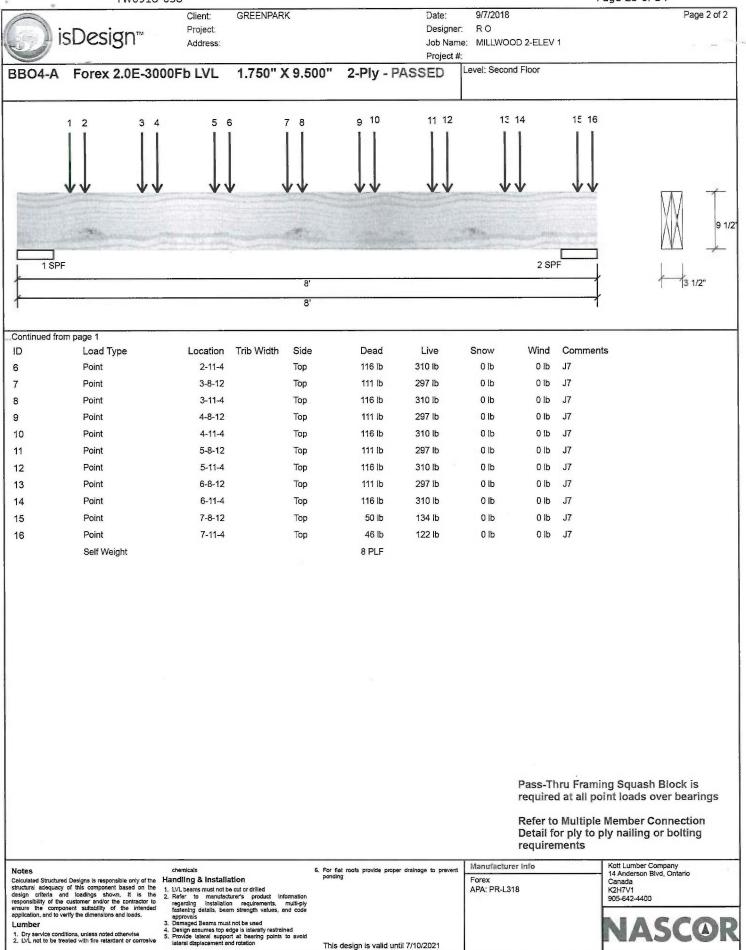




IN THE DESIGN OF THIS COMPONENT.

This design is

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive





Project: Address:

GREENPARK

Date: Designer: RO

9/7/2018

Job Name: MILLWOOD 2-ELEV 1

Page 1 of 1

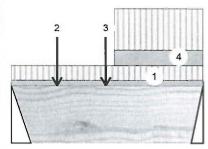
Project #:

Forex 2.0E-3000Fb LVL

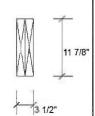
1.750" X 11.875"

2-Ply - PASSED

Level: Second Floor



1 Hanger (HGUS410) 2 Hanger (HUC410 (Min)) 3'3 3/4 3'3 3/4"



Member Info	rmation		
Туре:	Girder	Application:	Floor (Residential)
Plies:	2	Design Method:	LSD
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dond:	15 DOC		

Dead:	15 PSF										
Analysis Results											
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case					
Moment	246 ft-lb	1'7 1/2"	34261 ft-lb	0.007 (1%)	1.25D+1.5L	L					
Unbraced	246 ft-lb	1'7 1/2"	34261 ft-lb	0.007 (1%)	1.25D+1.5L	L					

246 lb 1'3 1/8" 11596 lb 0.021 (2%) 1.25D+1.5L L Shear Perm Defl in. 0.000 (L/999) 0 999.000 (L/0) 0.000 (0%) LL Defl inch 0.000 (L/999) 0 999.000 (L/0) 0.000 (0%) TL Defl inch 0.001 1'7 3/4" 0.145 (L/240) 0.010 (1%) D+L 1 (L/46486)

Design Notes

1'7 1/2" 34261 ft-lb 0.007 (1%) 1.25D+1.5L L

1 Fill all hanger nailing holes.

3 Multiple plies must be fastened together as per manufacturer's details. 4 Top loads must be supported equally by all plies.

2 Girders are designed to be supported on the bottom edge only.

- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on full section width

Unlactored Reactions UNPATTERINED ID (Upilit)									
Brg	Live	Dead	Snow	Wind					
4	150	92	Δ	0					

Unfortered Denstians UNDATTERNIED IL (Unlife)

2	169	88	0	0

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - Hanger	4.000"	3%	104 / 225	329	L	1.25D+1.5L
2 - Hanger	2.500"	6%	110 / 253	363	L	1.25D+1.5L



September 13, 2018

/ Lateral	sienderness ratio based	on full section width.							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 3-3-12	(Span)1-4-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	0-9-7		Near Face	26 lb	70 lb	0 lb	0 lb	J2
3	Point	1-7-8		Near Face	33 lb	35 lb	0 lb	PassaTh	ry-Framing Squash Block is at all point loads over bearings
4	Tie-In	1-9-4 to 3-3-12	(Span)	Тор	15 PSF	40 PSF	0 PSF	0 PSF	at all point loads over bearings
	Self Weight		3-11-13		10 PLF				Multiple Member Connection or ply to ply nailing or bolting ments

This design

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted atherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
 approvals
- approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

Manufacturer Info Forex APA: PR-L318

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Kott Lumber Company 14 Anderson Blvd, Ontario K2H7V1







GREENPARK

Project: Address: Date:

9/7/2018

RO Designer:

Job Name: MILLWOOD 2-ELEV 1

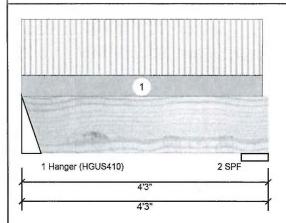
Project #:

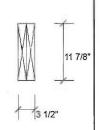
Forex 2.0E-3000Fb LVL

1.750" X 11.875"

2-Ply - PASSED

Level: Second Floor





Page 1 of 1

Member Inf	formation						Unfacto	red Read	tions U	NPATT	ERNED	lb (Uplift)	
Туре:	Girder		Applicat	ion:	Floor (Resident	tial)	Brg	Live	9	Dead		Snow	Wind
Plies:	2		Design	Method: I	_SD		1	3	5	33		0	0
Moisture Cond	lition: Dry		Building	Code:	NBCC 2010 / C	BC 2012	2	3:	5	34		0	0
Deflection LL:	360		Load Sh	naring:	No		-						
Deflection TL:	240		Deck:	1	Not Checked								
Importance:	Normal		Vibratio	n: 1	Not Checked								
General Load												*.	
Floor Live:	40 PSF						Bearing	s and Fa	ctored F	Reaction	าร		
Dead:	15 PSF						Bearing	Length	Cap.	React D/	LIb To	otal Ld. Case	Ld. Comb.
							1 - Hanger	4.000"	1%	41	/ 52	93 L	1.25D+1.5L
Analysis Re	sults						2-SPF	5.500"	1%	43	/ 53	95 L	1.25D+1.5L
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case						_	-
Moment	72 ft-lb	2' 3/4"	34261 ft-lb	0.002 (0%) 1.25D+1.5L	L						OFESSIO	NA.
Unbraced	72 ft-lb	2' 3/4"	34261 ft-lb	0.002 (0%) 1.25D+1.5L	L					1	PRO	12
Shear	36 lb	1'3 1/8"	11596 lb	0.003 (0%) 1.25D+1.5L	L					18	T.L. WIS	101
Perm Defl in	0.000 (L/999)	0	999.000 (L/0	0.000 (0%)						13		17 harman
LL Defl inch	0.000 (L/999)	0	999.000 (山	0.000 (0%)						9	T.L. VVI	SE E
TL Defl inch	0.000 (L/999)	0	999.000 (L/0	0.000 (0%)						1-	100083	000
Design Not	es						1				1	VII	1841
	er nailing holes.						7				1	2000	DO SHE
	designed to be su											WCE OF	ON
	es must be fastene nust be supported		•	sturer's details	S.						Sep	tember 1	
5 Top braced		equally by all	piles.								•		•
	ced at bearings.												
7 Lateral slen	derness ratio bas	ed on full sect	ion width.										
ID	Load Type		Location	Trib Width	Side	Dead	Liv	e S	now	Wind	Comme	nts	
1	Tie-In	0-0-0	to 4-1-14	(Span)0-10-1	Тор	15 PSF	40 PS	F 0	PSF	0 PSF			
	Self Weight					10 PLF							

Notes

Colculated Structured Designs is responsible only of the structural adequacy of this component based on hie design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the Intended application, and to verify the dimensions and loads. Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

Installation
 Installation
 Installation
 Installation
 Installation requirements, multi-ply testening details, beam strength values, and code approvals
 Damaged Beams must not be used
 Design essumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

This design

Manufacturer Info Forex APA: PR-L318

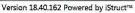
requirements

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Kott Lumber Company 14 Anderson Blvd, Ontario Canada K2H7V1 905-642-4400

Pass-Thru Framing Squash Block is required at all point loads over bearings Refer to Multiple Member Connection Detail for ply to ply nailing or bolting









GREENPARK

Project: Address:

Date: 9/7/2018

Designer: RO

Job Name: MILLWOOD 2-ELEV-1

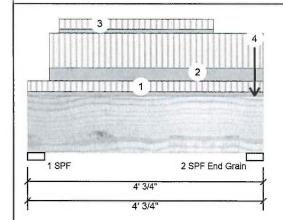
Project #:

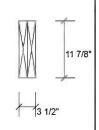
Forex 2.0E-3000Fb LVL

1.750" X 11.875"

2-Ply - PASSED

Level: Second Floor





Wind

0

0

Page 1 of 1

Туре:	Girder	Application:	Floor (Residential)
Plies:	2	Design Method:	LSD
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		

Bearings	and	Factored	Reactions

Live

643

885

Unfactored Reactions UNPATTERNED Ib (Uplift)

Dead

260

375

Snow

0

Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.
1-SPF	3.500"	17%	325 / 965	1289	L	1.25D+1.5L
2 - SPF End	3.500"	20%	469 / 1328	1797	L	1.25D+1.5L

Brg

1

2

Grain

Analysis Results

Member Information

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1215 ft-lb	2' 5/16"	34261 ft-lb	0.035 (4%)	1.25D+1.5L	L
Unbraced	1215 ft-lb	2' 5/16"	34261 ft-lb	0.035 (4%)	1.25D+1.5L	L
Shear	711 lb	2'10 1/8"	11596 lb	0.061 (6%)	1.25D+1.5L	L
Perm Defl in.	0.001 (L/34561)	2' 5/16"	0.120 (L/360)	0.010 (1%)	D	Uniform
LL Defl inch	0.003 (L/13799)	2' 5/16"	0.120 (L/360)	0.030 (3%)	L	L
TL Defl inch	0.004 (L/9862)	2' 5/16"	0.180 (L/240)	0.020 (2%)	D+L	L



- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on full section width.

September 13, 2018

1D	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind Comments
1	Tie-In	0-0-0 to 4-0-12	(Span)3-7-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF
2	Part. Uniform	0-4-8 to 4-0-12		Тор	90 PLF	240 PLF	0 PLF	0 PLF
3	Part. Uniform	0-6-8 to 3-2-8		Near Face	25 PLF	68 PLF	0 PLF	Pass_Fhru Framing Squash Block is
4	Point	3-11-0		Near Face	88 lb	169 lb	0 lb	required at all point loads over bearings
	Self Weight				10 PLF			Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on his design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads. Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code

approvals Damaged Beams must not be used

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex APA; PR-L318

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Kott Lumber Company 14 Anderson Blvd, Ontario Canada K2H7V1 905-642-4400



This design is

GREENPARK

Project:

Address:

9/7/2018 Date:

Designer: RO

Job Name: MILLWOOD 2-ELEV 1

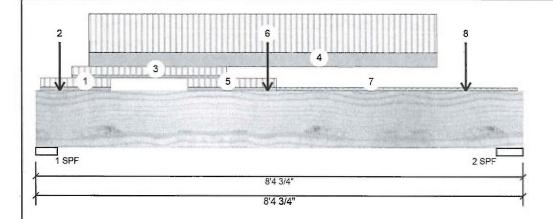
Project #:

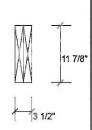
Forex 2.0E-3000Fb LVL F8-A

1.750" X 11.875"

2-Ply - PASSED

Level: Second Floor





Page 1 of 2

Member Inf <mark>o</mark> r	mation		Unfactored Reactions UNPATTERNED Ib (Uplift)							
Type:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snov	N	Wind
Plies:	2	Design Method:	LSD	1	1594		649		0	0
Moisture Conditio	n: Dry	Building Code:	NBCC 2010 / OBC 2012	2	1377		568		0	0
Deflection LL:	360	Load Sharing:	No							
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal	Vibration:	Not Checked							
General Load										
Floor Live:	40 PSF			Bearings	and Fact	ored R	eactions			
Dead:	15 PSF			Bearing	Length	Cap. I	React D/L lb	Total	Ld. Case	Ld. Comb.
				1-SPF	4.500"	33%	811 / 2391	3202	L	1.25D+1.5L
				2-SPF	5.500"	23%	710 / 2066	2776	L	1.25D+1.5L

-		_	
Ana	VSIS	Resu	lts.

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	5966 ft-lb	4'	34261 ft-lb	0.174 (17%)	1.25D+1.5L	L
Unbraced	5966 ft-lb	4'	31511 ft-lb	0.189 (19%)	1.25D+1.5L	L
Shear	3035 lb	7' 1/8"	11596 lb	0.262 (26%)	1.25D+1.5L	L
Perm Defl in.	0.016 (L/5638)	4' 3/4"	0.256 (L/360)	0.060 (6%)	D	Uniform
LL Defl inch	0.040 (L/2325)	4' 13/16"	0.256 (L/360)	0.150 (15%)	L	L
TL Defl inch	0.056 (L/1646)	4' 13/16"	0.384 (L/240)	0.150 (15%)	D+L	L



September 13, 2018

Design Notes 1 Girders are designed to be supported on the bottom edge only.

- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.

6. Lateral stenderness ratio based on full section width

O Lateral S	iendemess ratio based t	on run section width.							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-1-0 to 1-3-8	(Span)3-7-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	0-5-0		Near Face	77 lb	205 lb	0 lb	0 lb	J7
3	Part, Uniform	0-7-8 to 3-3-8		Far Face	25 PLF	68 PLF	0 PLF	0 PLF	
4	Part. Uniform	0-11-0 to 6-11-0		Near Face	115 PLF	308 PLF	0 PLF	0 PLF	
5	Tie-In	2-7-8 to 4-1-12	(Span)3-7-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
6	Point	4-0-0		Far Face	83 lb	150 lb	0 lb	0 lb	F6

Continued on page 2...

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the Intended application, and to verify the dimensions and loads. Lumber

Dry service conditions, unless noted otherwise LVL not to be treated with fire retardant or corrosi-

Handling & Installation

- 1. LVL beams must not be cut or drilled
 2. Refer to manufacturer's product information regarding installation requirements. multi-ply fastening details, beam strength values, and code approvals
 3. Demaged Beams must not be used
 4. Design assumes top edge is laterally restrained
 5. Provide lateral support at bearing points to evoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info Forex APA: PR-L318

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

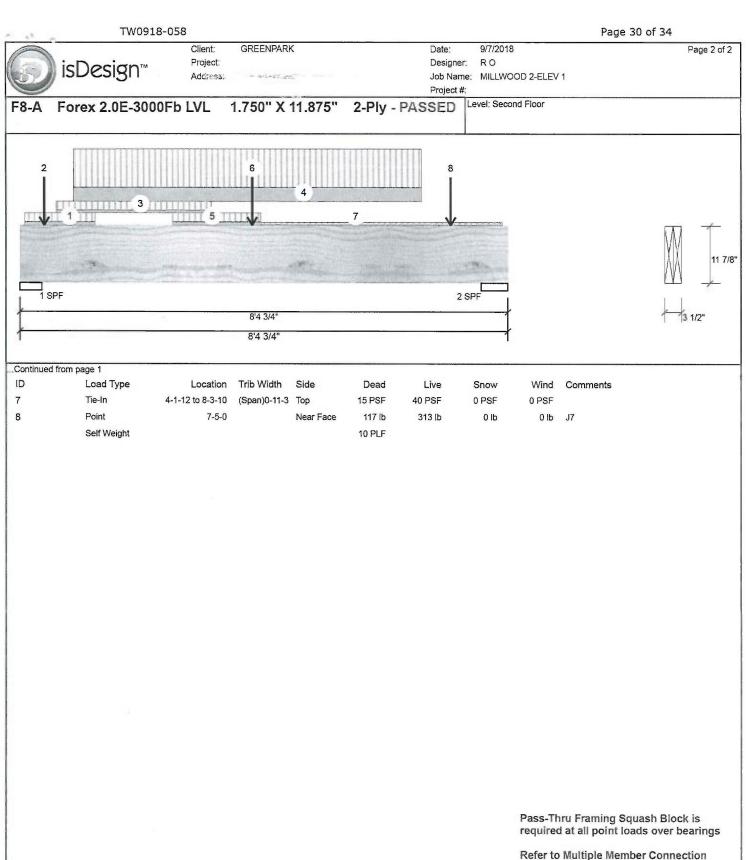
Kott Lumber Company 14 Anderson Blvd, Ontario Canada K2H7V1 905-642-4400



This design is







Detail for ply to ply nailing or bolting requirements

Notes

Calculated Structured Designs is responsible only of the structural edequacy of this component based on the design critieria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the Intended application, and to verify the dimensions and loads. Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or compsive

Handling & Installation

and ling & Installation

I/V beams must not be cut or drilled

Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvels

Damaged Beams must not be used

Design assumes top edge is laterally restrained

Provide lateral support at bearing points to avoid lateral displacement and rotation

This design is valid until 7/10/2021

Manufacturer Info

Forex APA: PR-L318

Kott Lumber Company 14 Anderson Blvd, Ontario Canada K2H7V1



Project: Address: **GREENPARK**

Date:

9/7/2018

Page 1 of 1

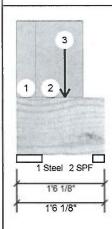
RO Designer: Job Name: MILLWOOD 2-ELEV 2

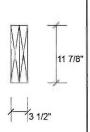
Project #:

Forex 2.0E-3000Fb LVL

1.750" X 11.875"

2-Ply - PASSED Level: Second Floor





Member Info	rmation	Unfactored Reactions UNPATTERNED Ib (Uplift)								
Type:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snov	w	Wind
Plies:	2	Design Method:	LSD	1	72		103		0	0
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	61		53		0	0
Deflection LL:	360	Load Sharing:	No							
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal	Vibration:	Not Checked							
General Load										
Floor Live:	40 PSF			Bearings a	nd Fact	ored l	Reactions			
Dead:	15 PSF			Bearing Le	ength	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.
				1 - Steel 5.3	250"	2%	129 / 108	237	L	1.25D+1.5L
				2-SPF 2	375"	3%	66 / 92	158	L	1.25D+1.5L

Analysis Results

	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case	_
	Moment	78 ft-lb	10"	33233 ft-lb	0.002 (0%)	1.25D+1.5L	L	
	Unbraced	78 ft-lb	10"	33233 ft-lb	0.002 (0%)	1.25D+1.5L	L	
	Shear	107 lb	1'4 3/8"	11248 lb	0.009 (1%)	0.9D+1.5L	L	
	Perm Defl in.	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)			
	LL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)			
l	TL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)			
-			***	***				-

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.

ENSED	PROFESSIONAL	ZGII.
LICEA	T.L. WISE 100083566	NEER
/3	SUD S	

September 13, 2018

6 Lateral:	slenderness ratio based o	on full section width.							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Part. Uniform	0-0-0 to 0-4-0		Тор	80 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
2	Part. Uniform	0-4-0 to 1-1-12		Тор	80 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
3	Point	0-10-0		Near Face	50 lb	133 lb	0 lb	0 lb	J10
	Self Weight				10 PLF				nru Framing Squash Block is d at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

Celculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- andling & Installation
 LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
 approvals
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info Forex APA: PR-L318

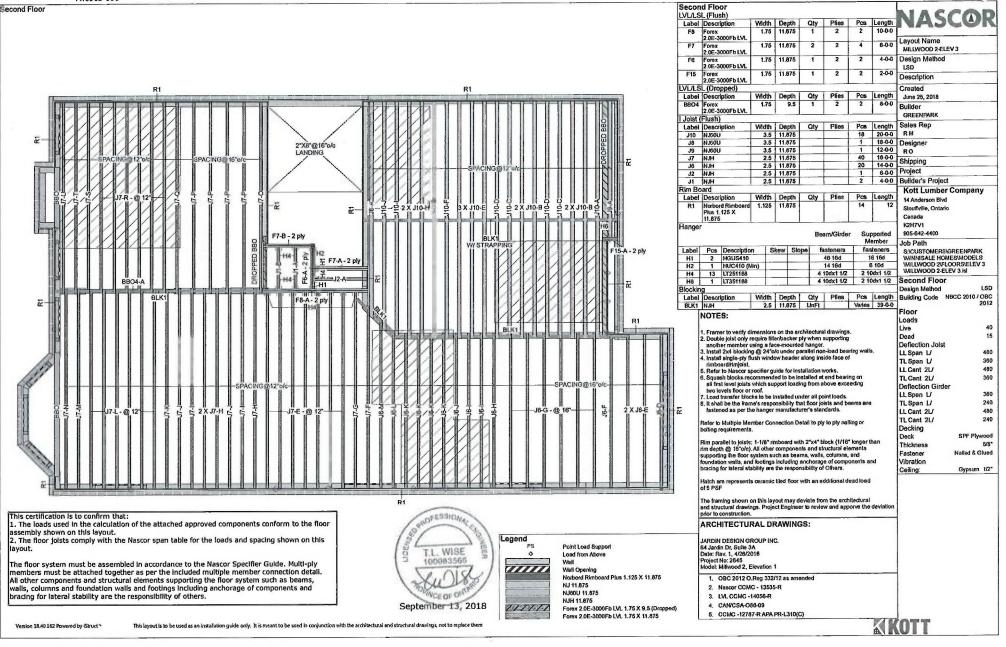
READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Kott Lumber Company 14 Anderson Blvd, Ontario Canada K2H7V1 905-642-4400





12 1





GREENPARK

Project: Address:

Date: Designer:

9/7/2018 RO

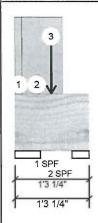
Job Name: MILLWOOD 2-ELEV 3

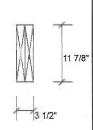
Project #:

Forex 2.0E-3000Fb LVL F15-A

1.750" X 11.875"

2-Ply - PASSED Level: Second Floor





Page 1 of 1

/lember Infor	mation			Unfactore	ed React	ions U	NPATTERNI	ED lb ((Uplift)	
Туре:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snov	w	Wind
Plies:	2	Design Method:	LSD	1	108		101		0	0
Moisture Conditio	n: Dry	Building Code:	NBCC 2010 / OBC 2012	2	65		41		0	0
Deflection LL:	360	Load Sharing:	No							
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal	Vibration:	Not Checked	1						
General Load		1000								
Floor Live:	40 PSF			Bearings a	and Fac	tored I	Reactions			
Dead:	15 PSF			Bearing L	ength	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
				1-SPF 5	5.250"	3%	126 / 162	288	L	1.25D+1.5L
				2-SPF 3	3.500"	2%	52 / 98	149	L	1.25D+1.5L

Analysis Results

Г	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case	_
	Moment	58 ft-lb	7 1/2"	34261 ft-lb	0.002 (0%)	1.25D+1.5L	L	
	Unbraced	58 ft-lb	7 1/2"	34261 ft-lb	0.002 (0%)	1.25D+1.5L	L	
l	Shear	190 lb	1'4 3/8"	11596 lb	0.016 (2%)	1.25D+1.5L	L	
l	Perm Defl in.	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)			
l	LL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)			
	TL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)			
۳								

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on full section width.

1860	PROFESSIONAL	Non
LICEA	T.L. WISE 100083566	EER
/3	SUNCE OF ONTH	8/

September 13, 2018

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Part. Uniform	0-0-0 to 0-1-8		Тор	80 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
2	Part. Uniform	0-1-8 to 0-9-12		Тор	80 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
3	Point	0-7-8		Far Face	65 lb	173 lb	0 lb	0 lb	J9
	Self Weight			10 PLF				Pass-Thru Framing Squash Block is required at all point loads over bearings	

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads. Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

andIling & Installation
LVL beams must not be cut or drilled
Rafer to manufacturer's product information
regarding installation requirements, mufti-pil
fastening details, beam strength values, and code
approvals
Damaged Beams must not be used
Design assumes top edge is laterally restrelaed
Provide lateral support at bearing points to avoid
lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info Forex APA: PR-L318

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Kott Lumber Company 14 Anderson Blvd, Ontario Canada K2H7V1 905-642-4400

