Layout Name

Design Method

Description

June 25, 2018

GREENPARK

Created

Builder

Sales Rep

Designer

Shipping

Builder's Project

14 Anderson Blvd

Stouffville, Ontario

Kott Lumber Company

S:\CUSTOMERS\GREENPARK

\MINNISALE HOMES\MODELS \MILLWOOD 2\FLOORS\ELEV 1 \MILLWOOD 2-ELEV 1.isl

Building Code NBCC 2010 / OBC

LSD

40

15

480

360

480

360

360

240

480

240

3/4"

SPF Plywood

Nailed & Glued

Project

Canada

K2H7V1

Job Path

905-642-4400

Deflection Joist LL Span L/

TL Span L/

LL Cant 2L/

TL Cant 2L/

LL Span L/

TL Span L/

LL Cant 2L/

TL Cant 2L/

Decking

Thickness

Fastener

Vibration

Deck

Deflection Girder

Floor Loads

Live

Dead

RM

RO

LSD

MILLWOOD 2-ELEV 1

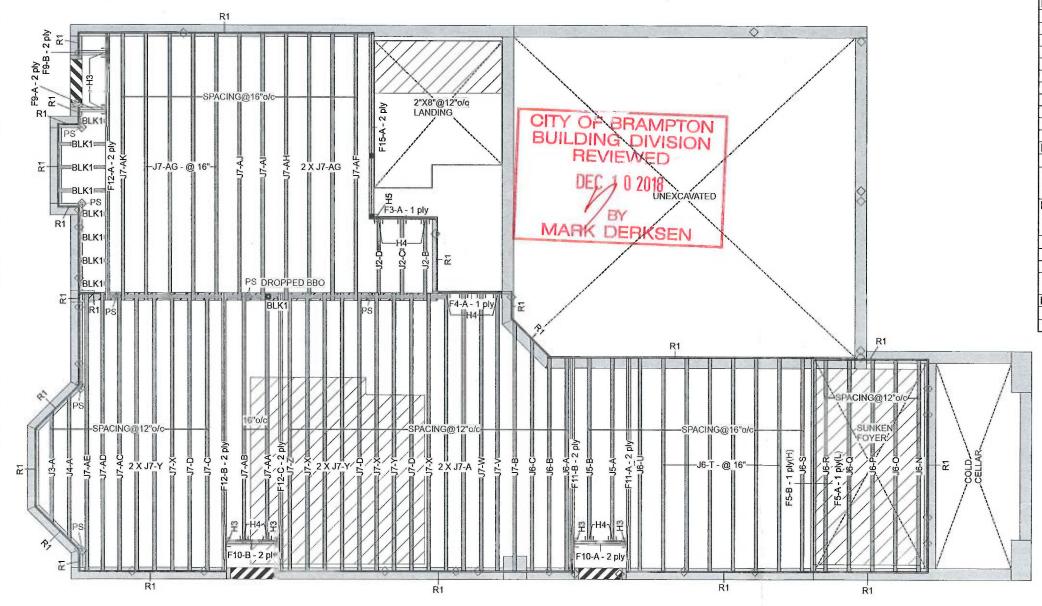
Pcs Length

12-0-0

Ground Floor

All work shall conform to the Ontario Building Code O. Reg. 332/12 as amended

Engineered floor joists shall be installed in accordance with the supplier's layout and specifications forming part of the permit drawings



This certification is to confirm that:

1. The loads used in the calculation of the attached approved components conform to the floor assembly shown on this layout. 2. The floor joists comply with the Nascor span table for the loads and spacing shown on this layout.

The floor system must be assembled in accordance to the Nascor Specifier Guide, Multi-ply members must be attached together as per the included multiple member connection detail.

All other components and structural elements supporting the floor system such as beams, walls, columns and foundation walls and footings including anchorage of components and bracing for lateral stability are the responsibility of others.



1. OBC 2012 O.Reg 332/12 as amended

2. Nascor CCMC - 13535-R

3. LVL CCMC -14056-R

4. CAN/CSA-O86-09

5. CCMC -12787-R APA PR-L310(C)

F4 1.75 11.875 6-0-0 2.0E-3000Fb LVL F3 1.75 11.875 Forex 4-0-0 2.0E-3000Fb LVL I Joist (Flush) Qty Plies 3 2 Label Description Width Depth Pcs Length F12 NJ 1.5 11.875 6 16-0-0 F11 NJ 1.5 11.875 4 14-0-0 F10 NJ 1.5 11.875 2 4 4-0-0 F9 NJ 1.5 11.875 2-0-0 J7 NJH 2.5 11.875 35 16-0-0 J6 NJH 16 14-0-0 2.5 11.875 J5 NJH 2.5 11.875 12-0-0 J4 NJH 2.5 11.875 1 10-0-0 J3 NJH 2.5 11.875 1 8-0-0 3 6-0-0 J2 NJH 2.5 11.875 Rim Board Pcs Length Label Description Width Depth Qty Plies Norbord Rimboar 1.125 11.875 12 Plus 1.125 X 11.875 Hanger

Width Depth

1.75 11.875

1.75 11.875

Qty Plies

Ground Floor LVL/LSL (Flush)

F5

F15

Label Description

Forex 2.0E-3000Fb LVL

Forex

2.0E-3000Fb LVL

					Beam/Girder	Supported Member
Label	Pcs	Description	Skew	Slope	fasteners	fasteners
НЗ	6	LT2-151188			4 10dx1 1/2	2 10dx1 1/2
H4	11	LT251188			4 10dx1 1/2	2 10dx1 1/2
H5	1	HUCQ1.81/9- SDS				

Blocking Label Descri BLK1 NJH

							Ground Floo	r
iption	Width	Depth	Qty	Plies	Pcs	Length	Design Method	
	2.5	11.875	LinFt		Varies	29-0-0	D. II.II O. I	h 1

NOTES:

I. Framer to verify dimensions on the architectural drawings. Double joist only require filler/backer ply when supporting another member using a face-mounted hanger.

Install 2x4 blocking @ 24"0/c under parallel non-load bearing walls.
 Install single-ply flush window header along inside face of

5. Refer to Nascor specifier guide for installation works.

6. Squash blocks recommended to be installed at end bearing on all first level joists which support loading from above exceeding two levels floor or roof. Load transfer blocks to be installed under all point loads.

. It shall be the frame's responsibility that floor joists and beams are fastened as per the hanger manufacturer's standards.

Refer to Multiple Member Connection Detail to ply to ply nailing or bolting requirements.

Rim parallel to joists: 1-1/8" rimboard with 2"x4" block (1/16" longer than rim depth @ 16"o/c). All other components and structural elements supporting the floor system such as beams, walls, columns, and foundation walls, and footings including anchorage of components and bracing for lateral stability are the responsibility of Others.

Hatch are represents ceramic tiled floor with an additional dead load

The framing shown on this layout may deviate from the architectural and structural drawings. Project Engineer to review and apporve the deviation prior to construction.

ARCHITECTURAL DRAWINGS:

JARDIN DESIGN GROUP INC. 64 Jardin Dr, Suite 3A Date: Rev. 1, 4/26/2018 Project No: 2645 Model: Millwood 2, Elevation 1

Legend

Point Load Support Load from Above Wall Opening

Norbord Rimboard Plus 1.125 X 11.875 NJ 11.875

NJH 11.875

Forex 2.0E-3000Fb LVL 1.75 X 11.875



Version 18.40.162 Powered by iStruct™

This layout is to be used as an installation guide only. It is meant to be used in conjunction with the architectural and structural drawings, not to replace them

MILLWOOD 2-ELEV 1

Kott Lumber Company

S:\CUSTOMERS\GREENPARK MINNISALE HOMESIMODELS

\MILLWOOD 2\FLOORS\ELEV 1

Building Code NBCC 2010 / OBC

LSD

2012

40

15

480

360

480

360

360

240

480

240

SPF Plywood

Nailed & Glued

Gypsum 1/2"

\MILLWOOD 2-ELEV 1.isl

Second Floor

Design Method

Deflection Joist

LL Span L/

TL Span L/

LL Cant 2L/

TL Cant 2L/

LL Span L/

TL Span L/

LL Cant 2L/

TL Cant 2L/

Decking

Thickness

Fastener

Vibration

Ceiling:

Deck

Deflection Girder

Floor

Loads

Live

Dead

Varies 39-0-0

Second Floor

This certification is to confirm that:

- 1. The loads used in the calculation of the attached approved components conform to the floor assembly shown on this layout.
- 2. The floor joists comply with the Nascor span table for the loads and spacing shown on this

The floor system must be assembled in accordance to the Nascor Specifier Guide. Multi-ply members must be attached together as per the included multiple member connection detail. All other components and structural elements supporting the floor system such as beams, walls, columns and foundation walls and footings including anchorage of components and



bracing for lateral stability are the responsibility of others. September 13, 2018 R1 2"X8"@16"o/c LANDING -22 2 X J9-C 6 6 2 X J9-C 3 X J9-E F7-B - 2 ply WISTRAPPING Label | Description ∓ F7-A - 2 ply BLK1 NJH BLK1 F8-A - 2 ply 2 X J7-H -J6-G - @ 16"-2 X J6-E

R1

Secon	d Flo	or									O = = = =	
LVL/LS	L (Flus	sh)									MIAC	
Label	Descri	ption	Width	De	pth	Q	ty	Plies	Pcs	Length	NAS	
F8	Forex 2.0E-3	DOOF6 LVL	1.75	11.	875	1		2	2	10-0-0		
F7	Forex 2.0E-3	000Fb LVL	1.75	11.	875	2	2	2	4	6-0-0	Layout Name MILLWOOD 2-ELE\	
F6	Forex 2.0E-3	000Fb LVL	1.75	11.	875	1		2	2	4-0-0	Design Method	
LVL/LS	L (Dro	pped)										
Label	Descri	ption	Width	De	pth	Q	ty	Plies	Pcs	Length	Description	
BBO4	Forex 2.0E-3	000Fb LVL	1.75		9.5	1		2	2	8-0-0	Created June 25, 2018	
I Joist (Flush)										Builder	
Label	Descri	ption	Width	De	pth	Q	ty	Plies	Pcs	Length	GREENPARK	
J9	NJ60U		3.5	11.	875				19	20-0-0		
J8	NJ60U		3.5	11.3	875				1	18-0-0	Sales Rep	
J7	NJH		2.5	11.	875				40	16-0-0	RM	
J6	NJH		2.5	11.	875				20	14-0-0	Designer	
J2	NJH		2.5	11.	875				1	6-0-0	RO	
J1	NJH		2.5	11.	875				2	4-0-0	Shipping	
Rim Bo	ard										Project	
Label	Descri	ption	Width	De	pth	Q	ty	Plies	Pcs	Length		
R1		d Rimboard	1.125	11.	875				14	12	Builder's Project	
	Plus 1. 11.875	125 X									Kott Lumber (
Hanger	•						Bea	am/Girder		oported ember	14 Anderson Blvd Stouffville, Ontario Canada	
Label	Pcs	Descriptio	n s	Skew	Slop	oe	fa	steners	fas	teners	K2H7V1	
H1	2	HGUS410				1	_	46 16d		6 16d	905-642-4400	
H2	1	HUC410 (N	in)					14 16d	6	3 10d	Job Path	
H4	13	LT251188					4	10dx1 1/2	2 10	dx1 1/2	S:\CUSTOMERS\G	
Blockin	g										MINNISALE HOME	
Label	el Description		Width	De	pth	Q	ly	Plies	Pcs	Length	\MILLWOOD 2\FLO	
	bot Becompain			1			_	1		+	MILLWOOD 2-ELE	

NOTES:

- . Framer to verify dimensions on the architectural drawings.
- 2. Double joist only require filler/backer ply when supporting another member using a face-mounted hanger.
- Install 2x4 blocking @ 24"o/c under parallel non-load bearing walls.
- Install single-ply flush window header along inside face of rimboard/rimioist.

2.5 11.875 LinFt

- Refer to Nascor specifier guide for installation works.
- 6. Squash blocks recommended to be installed at end bearing on all first level joists which support loading from above exceeding two levels floor or roof.
- 7. Load transfer blocks to be installed under all point loads.
- 3. It shall be the frame's responsibility that floor joists and beams are fastened as per the hanger manufacturer's standards.

Refer to Multiple Member Connection Detail to ply to ply nailing or bolting requirements.

Rim parallel to joists: 1-1/8" rimboard with 2"x4" block (1/16" longer than rim depth @ 16"o/c). All other components and structural elements supporting the floor system such as beams, walls, columns, and foundation walls, and footings including anchorage of components and bracing for lateral stability are the responsibility of Others.

Hatch are represents ceramic tiled floor with an additional dead load

The framing shown on this layout may deviate from the architectural and structural drawings. Project Engineer to review and apporve the deviation prior to construction

ARCHITECTURAL DRAWINGS:

JARDIN DESIGN GROUP INC. 64 Jardin Dr. Suite 3A Date: Rev. 1, 4/26/2018 Model: Millwood 2, Elevation 1

Legend

//////

11/1/1

Point Load Support Load from Above Wall

Wall Opening

Norbord Rimboard Plus 1.125 X 11.875

NJ 11.875 NJ60U 11.875 NJH 11.875

Forex 2.0E-3000Fb LVL 1.75 X 9.5 (Dropped) Forex 2.0E-3000Fb LVL 1.75 X 11.875



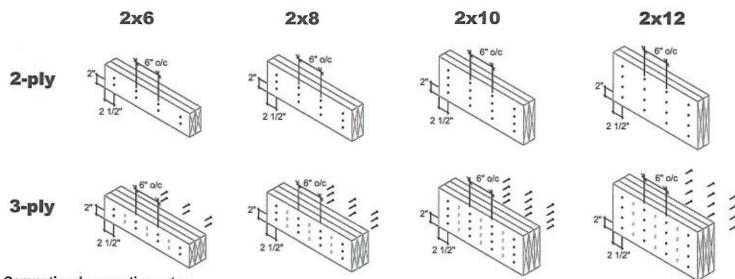
- 1. OBC 2012 O.Reg 332/12 as amended
- 2. Nascor CCMC 13535-R
- 3. LVL CCMC -14056-R
- 4. CAN/CSA-O86-09
- 5. CCMC -12787-R APA PR-L310(C)

Version 18.40.162 Powered by iStruct™

This layout is to be used as an installation guide only. It is meant to be used in conjunction with the architectural and structural drawings, not to replace them

MULTIPLE MEMBER CONNECTIONS

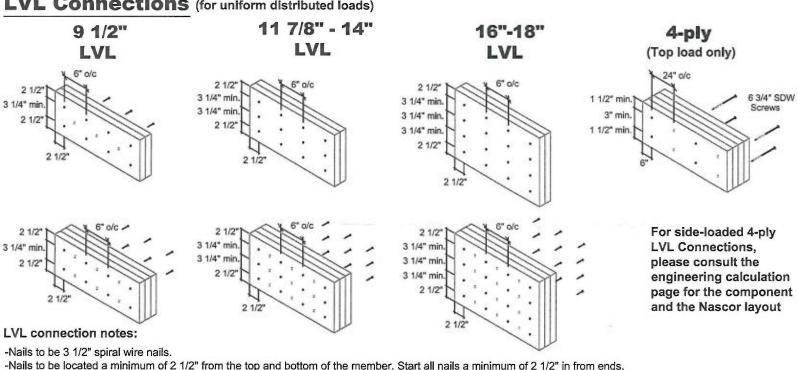
Conventional Connections (for uniform distributed loads)



Conventional connection notes:

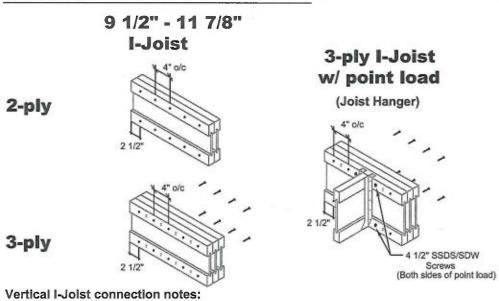
- -Nails to be 3" 10d spiral wire nails.
- -Nails to be located a minimum of 2" from the top and bottom of the member. Start all nails a minimum of 2 1/2" in from ends.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail driven from the opposite side.

LVL Connections (for uniform distributed loads)



- -Nails to be located a minimum of 2 1/2" from the top and bottom of the member. Start all nails a minimum of 2 1/2" in from ends.
- -Minimum 3 1/4" spacing between rows.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail or screw driven from the opposite side.

Vertical I-Joist Connections (for uniform distributed loads)



- -Nails to be 3" spiral wire nails.
- -Nails to be located at centre of top and bottom flanges. Start all nails a minimum of 2 1/2" in from ends.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail driven from the opposite side.

MULTI-PLY CONNECTION DETAILS

> Date: November 30, 2016 Scale: NTS

KOTT 3228 Moodie Drive Ottawa, ON **K2H 7V1** Ph: 613-838-2775

Fx: 613-838-4751

Engineering Note Page (ENP-2)

REVISION 2009-10-09

Please read all notes prior to installation of the component

DESIGN INFORMATION

This building component is certified as an individual component for the loads and conditions shown on the calculation and drawing page.

The responsibility of the undersigned engineer is <u>only</u> limited to the calculation of this building component for the loads and conditions shown on this drawing.

The responsibility of the undersigned is limited to the verification of the structural capacity of the NASCOR floor joists and LVL beams based on placement as shown on the layout. The loads applied are limited to the gravity effects of the specified loads. The structural integrity of the building and the effect of wind, uplift, seismic, lateral or other forces, calculation of adequate support and anchorage of components, as well as the dimensions and design loads used to calculate components are the responsibility of the overall building designer.

Floor joists and OSB rim board are designed to carry uniformly distributed loads only. Point loads should be transferred through the floor cavity with squash blocks. Structural elements such as walls, posts, connectors, and squash blocks are the responsibility of the overall building designer.

The undersigned engineer disclaims any responsibility for damages as a result of being furnished faulty or incorrect information, specifications and/or designs.

Installation of NASCOR joists is to be carried out in accordance with the current edition of the manufacturer's approved literature available at http://www.nascor.ca.

CODE

This building component is designed in accordance with the National Building Code of Canada, the Ontario Building Code, CCMC and Canadian Standards Association guidelines.

COMPONENT

- 1. The building component used in construction must be the same as indicated on the drawings.
- 2. The building component must be installed and assembled as per specification shown on the drawing and in accordance with the manufacturer's assembly and installation.
- 3. Members consisting of multiple plies must be connected as per the document "Multi-ply Connection Details".
- 4. Pass-thru squash block framing is required at all point loads over bearings.

HANDLING AND INSTALLATION

Do not drill any hole, cut or notch a certified building component without a written preauthorization.



MULTIPLE MEMBER CONNECTIONS

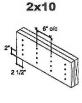
Conventional Connections (for uniform distributed loads)

2-ply

2x6









3-ply









Conventional connection notes:

-Nails to be 3" 10d spiral wire nails.

-Nails to be located a minimum of 2" from the top and bottom of the member. Start all nails a minimum of 2 1/2" in from ends.

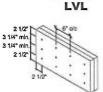
-Number of rows and spacing as per details shown, unless noted otherwise.

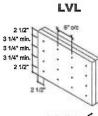
-"X" represents nail driven from the opposite side.

11 7/8" - 14"

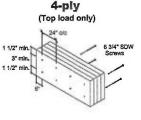
LVL Connections (for uniform distributed loads)

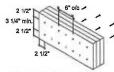
9 1/2" 2 1/2

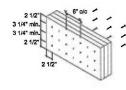


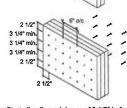


16"-18"









For side-loaded 4-ply LVL Connections. please consult the engineering calculation page for the component and the Nascor layout

LVL connection notes:

- -Nails to be 3 1/2" spiral wire nails.
- -Nais to be 3 1/2" spiral wire nails.

 -Nais to be located a minimum of 2 1/2" from the top and bottom of the member. Start all nails a minimum of 2 1/2" in from ends.

 -Minimum 3 1/4" spacing between rows.

 -Number of rows and spacing as per details shown, unless noted otherwise.

 -"X" represents nail or screw driven from the opposite side.

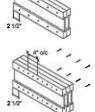
9 1/2" - 11 7/8"

Vertical I-Joist Connections (for uniform distributed loads)

I-Joist

2-ply

3-ply





3-ply I-Joist

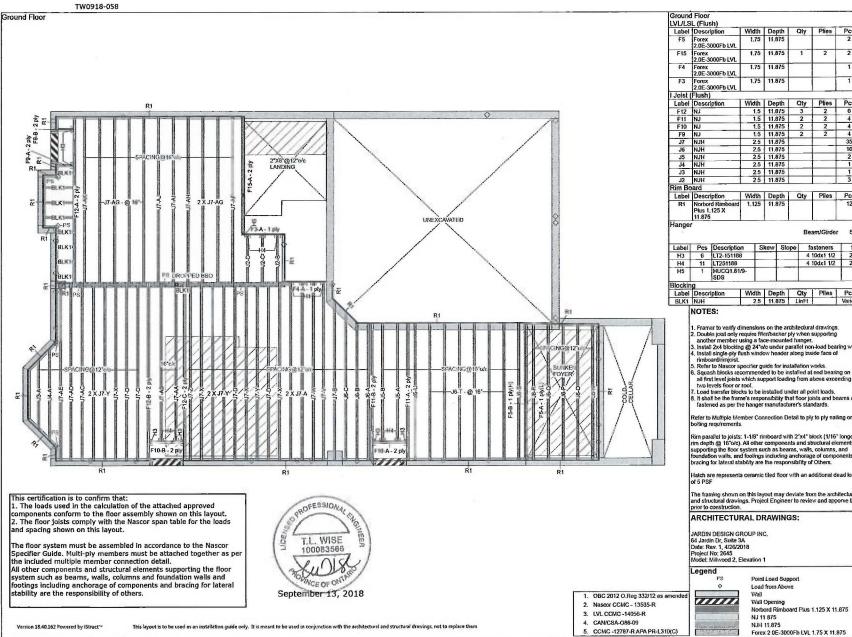
4 1/2" SSDS/SDW Screws (Both sides of point load)

Vertical I-Joist connection notes:

- -Nails to be 3" spiral wire nails,
 -Nails to be located at centre of top and bottom flanges. Start all nails a minimum of 2 1/2" in from ends.
- -Number of rows and spacing as per details shown, unless noted otherwise.
 "X" represents nail driven from the apposite side.

KOTT 3228 Moodle Drive Ottawa, ON K2H 7V1 Ph: 613-838-2775

MULTI -PLY CONNECTION DETAILS



										Page 3 of 3	4
Ground	Floor										
LVL/LS	L (Flus	sh)								MIAC	COR
	Descr	ption	Widt		pth	Qty	Plies	Pcs	Length	NAS	
F5	Forex		1.7	5 11	875			2	14-0-0		
F15	Forex	000Fb LVL	1.7	5 11	875	1	- 2	2	12-0-0	Layout Name	
FIS	2.0E-3	OOOF6 LVL	1.4	3 "	613				12-0-0	MILLWOOD 2-E	LEV 1
F4	Forex		1.7	5 11	875			1	6-0-0	Design Method	1
	2.0E-3	000Fb LVL								LSD	
F3	Forex	00000 110	1.7	5 11	875			1	4-0-0	Description	
I Joist (OCOF _b LVL					_	└	1		
	Descr		Widi	h I D	nth	Qty	Plies	Pcs	Length	Created	
F12	NJ	hini			9th 875	3	2	6	16-0-0	June 25, 2018	
F11	NJ				875	2	2	4	14-0-0	Builder	
F10	NJ				875	2	2	4	4-0-0	GREENPARK	
F9	NJ		1	5 11	875	2	2	4	2-0-0	Sales Rep	
J7	HLM				875			35	16-0-0	RM	
J6	NJH				875	_		16	14-0-0	Designer	
J5	NJH				875		_	2	12-0-0	RO	
J4 J3	NJH				.875 .875		+	1	10-0-0 8-0-0	Shipping	
J2	NJH				875	-	-	3	6-0-0	Project	
Rim Bo				5 [11	0.0	_			1 000	Builder's Proje	rt
	Descr	intion	Widi	h D	pth	Qty	Plies	Pcs	Length	Kott Lumbe	
R1		d Rimboard	1.12		875	,		12	12	1	
	Plus 1.	125 X		-						14 Anderson Blv	-
	11.875						J			Stoutfville, Ontai	rio
Hange	r					_		_		Canada K2H7V1	
1						В	eam/Girde		pported lember	905-642-4400	
Label	Pcs	Description	n	Skew	Slo	na	fasteners		steners		
H3	6	LT2-1511B		UNGIT	0.0		10dx1 1/2		0dx1 1/2	Job Path s:customers	NORTHIBLEIC
H4	11	LT251188	_		+-		1 10dx1 1/2		0dx1 1/2	VMINNISALE HO	
H5	1	HUCQ1.81	/9-		1					WILLWOOD 2VF	LOORSIELEV 1
		SDS								WILLWOOD 2-E	ELEV 1.isl
Blockin					8		C	-	Ir o	Ground Floo	
	Descr	iption	Wid		pth	Qty	Plies	Pcs	Length	Design Method	LSD
BLX1	_		2	5 11	875	LinFt		Varies	29-0-0	Building Code	NBCC 2010 / OBC
	NOTE	S:								Floor	2012
										Loads	
	1. Fram	er to verify d le joist only	mensi	ons on	the ar	chitectu	ral drawing	S.		Live	40
- 1	anoth	er memher i	usina a	face-m	counte	d hanos	of.			Dead	15
	3. Instal	2x4 blockin single-ply f	g @ 2	l'o/c u	der p	ara#el n	on-load be	aring wal	fs.	Deflection Jois	
1	4. Instal	l single-ply f ard/rimjoist.	lush wi	ndow h	eader	along a	nside face o	of		LL Span L/	480
1	5. Refer	to Nascor s	pecifie	ravide	for ins	tallation	works.			TL Span L/	360
- 1	6. Soua	sh blocks re	comme	nded to	be in	stalled:	at end bear	ing on		LL Cant 2L/	480
- 1	all firs	at level joists	which	suppor	load	ng from	above exc	eeding		TL Cant 2L/	360
	7. Load	transfer blo	cks to b	e insta	led ur	ider all i	point loads.			Deflection Gird	er
	8. It sha	# be the fran	ne's re	sponsic	ility th	at floor	joists and b	eams an	e	LL Span L/	360
	faster	red as per th	ne hang	jer mar	ufach	irer's st	andards.			TL Span L/	240
- 1	Refer to	Multiple Me	mber (Connec	ion D	etail to r	oly to ply na	iting or		LL Cant 2L/	480
		equirements						-		TL Cant 2L/	240
	Dire nor	allel to joists	- 1 1/0	· rimba	and will	h 254"	block /1/16	looge	than	Decking	
	rim dep	th@ 16*o/c)	. All oll	ser corr	DOLLGI	its and	structural e	lements	UIDII	Deck	SPF Plywood
- 1	supporti	ing the floor	system	such a	s bea	ms, wat	is, columns	, and		Thickness	3/4
- 1	foundati	ion walls, an for lateral st	d footi	ngs ind	uding	anchora	age of comp	onents a	ind	Fastener	Natled & Glued
- 1	Dracing	ios laterai si	antity o	ne me	espoi	MILITY	ui Culeis.			Vibration	
	Hatch a of 5 PSI	re represent F	s cerar	nic tiled	floor	with an	additional	dead load	i		
- 1											
- 1	and stru	ning shown ctural drawi construction	ngs. Pr	layout oject E	nay d	eviate fi er to rev	rom the arc iew and ap	hitectura porve the	deviation		
	and stru prior to	ctural drawi	ngs, Pr	oject E	ngine	er to rev	rom the arc iew and ap	hitectura porve the	deviation		
	ARCH JARDIN 64 Jardi	ctural drawi construction HTECTU I DESIGN G in Dr. Suite 3	RAL ROUP	oject E DRAV	ngine	er to rev	rom the arc	hitectura porve the	l deviation		
	ARCH JARDIN 64 Jardi Date: R Project	ctural drawi construction IITECTU	RAL ROUP BA 018	DRAV	ngine	er to rev	rom the arc	hitectura porve the	l e deviation		
	ARCH JARDIN 64 Jardi Date: R Project	ictural drawiconstruction IITECTU IDESIGN G in Dr., Suite 3 ev. 1, 4/26/2 No: 2645 Millwood 2, 1	RAL ROUP SA 018	DRAV	VINC	SS:	rom the arc	hitectura porve the	deviation		



GREENPARK

Project:

Address:

Date: Designer:

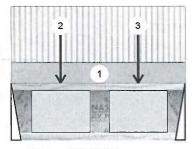
9/7/2018

Job Name: MILLWOOD 2-ELEV 1

Project #:

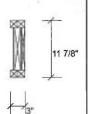
11.875" 2-Ply - PASSED F10-A

Level: Ground Floor



1 Hanger (LT2-151188) 2 Hanger (LT2-151188) 3'

15 PSF



Ld. Comb.

1.25D+1.5L

1.25D+1.5L

Page 1 of 1

Member Infor	mation			Unfacto	red Reaction	S UNPATTER	NED lb (Uplif	t)
Type:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind
Plies:	2	Design Method:	LSD	1	282	106	0	0
Moisture Condition	n: Dry	Building Code:	NBCC 2010 / OBC 2012	2	287	108	0	0
Deflection LL:	360	Load Sharing:	No					
Deflection TL:	240	Deck:	Not Checked					
Importance:	Normal	Vibration:	Not Checked					
General Load								
Floor Live:	40 PSF			Bearing	s and Factore	d Reactions		

Bearing Length

Hanger

2 -

2.000"

2.000"

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	401 ft-lb	1'4 1/2"	9020 ft-lb	0.044 (4%)	1.25D+1.5L	L
Unbraced	401 ft-lb	1'4 1/2"	5749 ft-lb	0.070 (7%)	1.25D+1.5L	L
Shear	558 lb	2'10 3/4"	3400 lb	0.164 (16%)	1.25D+1.5L	L
Perm Defl in.	0.001 (L/38142)	1'5 9/16"	0.093 (L/360)	0.010 (1%)	D	Uniform
LL Defl inch	0.002 (L/14284)	1'5 1/2"	0.093 (L/360)	0.030 (3%)	L	L
TI Deflinch	0.003	1'5 9/16"	0.140 (1/240)	0.020 (2%)	D+L	L

IL Deti inch (L/10392)

Hanger T.L. WISE 100083566 100083566

Cap. React D/L lb

20%

21%

132 / 423

135 / 431

WCE OF ONT September 13, 2018

Total Ld. Case

555 L

566 L

Design Notes

Dead:

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top flange braced at bearings.

6 Bottom	nange praced at bearings	i.					
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow
1	Tie-In	0-0-0 to 3-0-0	(Span)1-9-8	Тор	15 PSF	40 PSF	0 PSF
2	Point	0-10-4		Far Face	87 lb	233 lb	0 lb
3	Point	2-2-4		Far Face	86 lb	229 lb	0 lb

0 PSF Pass-Thru Framing Squash Block is required aball point loads over bearings

Comments

Wind

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 Usist not to be treated with fire retardant or corrosive

Handling & Installation

- Inditing & Installation
 Librist flanges must not be cut or drilled
 Refer to latest copy of the Librist product information
 details for framing details, stiffener tables, web hote
 chart, bridging details, multi-ply fastening details and
 handing/enction details
 Damaged Librist must not be used
- Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.
- Provide lateral support at bearing points to avoid lateral displacement and rotation
- s. Provide lateral support at obstaing points to avoid lateral displacement and rotation

 6. Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches

 7. For flat roofs proponding

 READ ALL NOTES ON 1

This design

Manufacturer Info Nascor by Kott

READ ALL NOTES ON THIS PAGE AND ON THE

ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Kott Lumber Company 14 Anderson Blvd, Ontario Canada K2H7V1 905-642-4400







GREENPARK

Project:

Address:

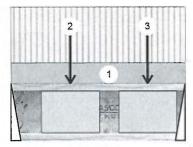
Date: Designer: 9/7/2018

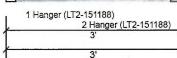
Page 1 of 1

Job Name: MILLWOOD 2-ELEV 1

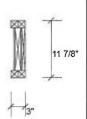
2-Ply - PASSED F10-B NJ 11.875"

Level: Ground Floor





15 PSF



Member Info	rmation			Unfacto	red Reaction	S UNPATTER	NED lb (Uplif	t)
Туре:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind
Plies:	2	Design Method:	LSD	1	343	129	0	0
Moisture Condit	ion: Dry	Building Code:	NBCC 2010 / OBC 2012	2	404	152	0	0
Deflection LL:	360	Load Sharing:	No					
Deflection TL:	240	Deck:	Not Checked					
Importance:	Normal	Vibration:	Not Checked					
General Load								
Floor Live:	40 PSF			Bearing	s and Factore	d Reactions		

Analysis Res	analysis Results										
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case					
Moment	583 ft-lb	1' 1/4"	9020 ft-lb	0.065 (6%)	1.25D+1.5L	L					
Unbraced	583 ft-lb	1' 1/4"	5749 ft-lb	0.101 (10%)	1.25D+1.5L	L					
Shear	788 lb	2'10 3/4"	3400 lb	0.232 (23%)	1.25D+1.5L	L					
Perm Defl in.	0.001 (L/27610)	1'1 5/16"	0.093 (L/360)	0.010 (1%)	D	Uniform					
11 Doff inch	0.003	11 5/16"	0.003 (1./360)	0.030 (3%)	T.	1					

/ li lai y SiS	/ IOLGGI	Loodaga	, morred	Japaony	Collins.	0000
Moment	583 ft-lb	1' 1/4"	9020 ft-lb	0.065 (6%)	1.25D+1.5L	L
Unbraced	583 ft-lb	1' 1/4"	5749 ft-lb	0.101 (10%)	1.25D+1.5L	L
Shear	788 lb	2'10 3/4"	3400 lb	0.232 (23%)	1.25D+1.5L	L
Perm Defl in.	0.001 (L/27610)	1'1 5/16"	0.093 (L/360)	0.010 (1%)	D	Uniform
LL Defl inch	0.003 (L/10370)	1'1 5/16"	0.093 (L/360)	0.030 (3%)	L	L
TL Defl inch	0.004 (L/7538)	1'1 5/16"	0.140 (L/240)	0.030 (3%)	D+L	L
					•	

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.	
1 - Hanger	2.000"	25%	161 / 514	675	L	1.25D+1.5L	
2 - Hanger	2.000"	29%	189 / 606	795	L	1.25D+1.5L	



Dead:

1 Fill all hanger nailing holes.

- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top flange braced at bearings.



e Rottom	nange braced at bearings	5,							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 3-0-0	(Span)1-9-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	1-0-4		Far Face	130 lb	346 lb	0 lb	0 lb	J7
3	Point	2-4-4		Far Face	110 lb	293 lb	0 lb		ru;Framing Squash Block is I at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 Upoist not to be treated with fire retardant or corrosive

Handling & Installation

4andling & Installation
I Joist flanges must not be cut or drilled
Refer to latest copy of the Joist product information details for framing details, stiffener tables, web hole teart, bridging details, multi-phy fasterling details and handling/erection details
Demaged Joists must not be used
Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

Previde lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeness for point load as shown Minimum point load bearing length>= 3.5 inches
 For flat roofs prompting READ ALL NOTES ON The state of the state of

This design

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Manufacturer Info Nascor by Kott

Kott Lumber Company 14 Anderson Blvd, Ontario Canada K2H7V1 905-642-4400







Client:

GREENPARK

Project: Address:

9/7/2018

RO

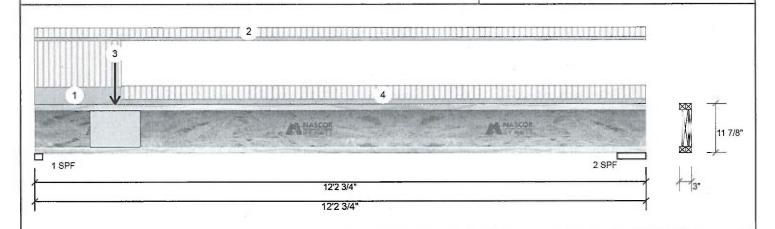
Designer: Job Name: MILLWOOD 2-ELEV 1

Project #:

Date:

2-Ply - PASSED NJ 11.875"

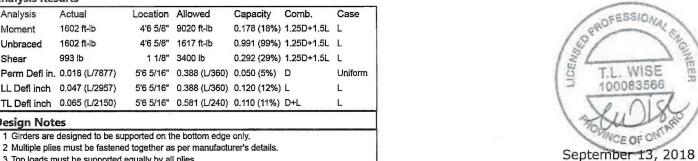
Level: Ground Floor



Member Infor	mation		4	Unfactored Reactions UNPATTERNED Ib (Uplift)					
Type:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind	
Plies:	2	Design Method:	LSD	1	512	192	0	0	
Moisture Condition	n: Dry	Building Code:	NBCC 2010 / OBC 2012	2	243	91	0	0	
Deflection LL:	360	Load Sharing:	No						
Deflection TL:	240	Deck:	Not Checked						
Importance:	Normal	Vibration:	Not Checked						
General Load									
Floor Live:	40 PSF			Bearings	and Facto	ored Reactions			
Dead:	15 PSF			Bearing I	Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.	
				1-SPF	1.875"	38% 240 / 767	1008 L	1.25D+1.5L	
				2-SPF 6	6.875"	14% 114 / 365	479 L	1.25D+1.5L	

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1602 ft-lb	4'6 5/8"	9020 ft-1b	0.178 (18%)	1.25D+1.5L	L
Unbraced	1602 ft-lb	4'6 5/8"	1617 ft-lb	0.991 (99%)	1.25D+1.5L	L
Shear	993 lb	1 1/8"	3400 lb	0.292 (29%)	1.25D+1.5L	L
Perm Defl in.	0.018 (L/7877)	5'6 5/16"	0.388 (L/360)	0.050 (5%)	D	Uniform
LL Defl inch	0.047 (L/2957)	5'6 5/16"	0.388 (L/360)	0.120 (12%)	L	L
TL Defl inch	0.065 (L/2150)	5'6 5/16"	0.581 (L/240)	0.110 (11%)	D+L	L
Design Note	es					



3 Top loads must be supported equally by all plies.

4 Top flange must be laterally braced at a maximum of 5'7" o.c.

5 Bottom flange braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-8-14	(Span)3-3-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 12-2-12	(Span)0-7-12	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	1-7-6		Far Face	108 lb	287 lb	0 lb	0 lb	F10
4	Tie-In	1-8-14 to 12-2-12	(Span)0-11-4	Тор	15 PSF	40 PSF	0 PSF		ru Framing Squash Block is at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 Usist not to be treated with fire retardant or corrosive

Handling & Installation

andling & Installation.

Loist flanges must not be cut or drilled.
Refer to latest copy of the Joist product information details for framing details, stiffener tables, web hole-thart. bridging details, multiply fastelning details and handling/erection details.

Damaged Joists must not be used.
Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

This design is

Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 For flat roofs pro

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Manufacturer Info

Nascor by Kott





Client:

GREENPARK

Project:

Address:

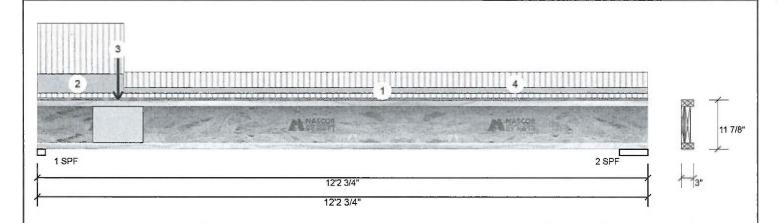
9/7/2018 Date: RO Designer:

Job Name: MILLWOOD 2-ELEV 1

Project #:

NJ 11.875" 2-Ply - PASSED F11-B

Level: Ground Floor



Member Info	rmation			Unfactore	ed Reaction	ns UNPATTERNI	D Ib (Uplift)	
Type:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind
Plies:	2	Design Method:	LSD	1	471	177	0	0
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	206	77	0	0
Deflection LL:	360	Load Sharing:	No					
Deflection TL:	240	Deck:	Not Checked					
Importance:	Normal	Vibration:	Not Checked	1				
General Load								
Floor Live:	40 PSF			Bearings	and Facto	red Reactions		
Dead:	. 15 PSF			Bearing L	ength.	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
				1 - SPF 1	.875"	35% 221 / 707	928 L	1.25D+1.5L

2 - SPF 6.875"

12%

96 / 308

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1410 ft-lb	4'3 1/4"	9020 ft-lb	0.156 (16%)	1.25D+1.5L	L
Unbraced	1410 ft-lb	4'3 1/4"	1412 ft-lb	0.998 (100%)	1.25D+1.5L	L
Shear	915 lb	1 1/8"	3400 lb	0.269 (27%)	1.25D+1.5L	L
Perm Defl in.	0.015 (L/9004)	5'5 11/16"	0.388 (L/360)	0.040 (4%)	D	Uniform
LL Defl inch	0.041 (L/3379)	5'5 11/16"	0.388 (L/360)	0.110 (11%)	L	L
TL Defl inch	0.057 (L/2457)	5'5 11/16"	0.581 (L/240)	0.100 (10%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 5'11" o.c.

5 Rottom flange braced at hearings



1.25D+1.5L

405 L

September 13, 2018

3 BOROTT	nange braced at bearing	gs.			1				
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 12-2-12	(Span)0-3-12	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 1-8-14	(Span)3-3-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	1-7-6		Near Face	106 lb	282 lb	0 lb	0 lb	F10
4	Tie-In	1-8-14 to 12-2-12	(Span) 0-11-12	Тор	15 PSF	40 PSF	0 PSF		ru Framing Squash Block is I at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Colculated Structured Designs is responsible only of the structural edequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 Usist not to be treated with fire retardant or corrosive

Handling & Installation

Handling & Installation

1. Iblist flages must not be cut or drilled

2. Refer to latest copy of the Libits product information
details for framing details, stiffener tables, web hole
chart, bridging details, multi-py bastening details and
handling/erection details

3. Damaged bloists must not be used

Dasign assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

This design is

5. Provide lateral support at bearing points to avoid lateral displacement and rotation
6. Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
7. For flat roofs pro

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Manufacturer Info

Nascor by Kott



Client:

GREENPARK

Project:

Address:

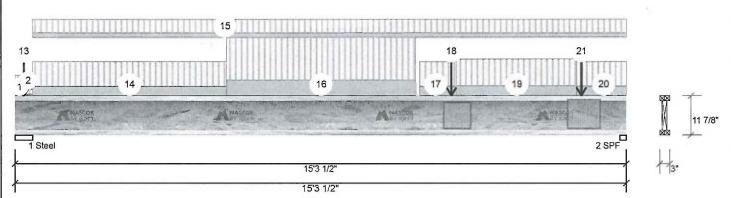
Date: Designer:

9/7/2018 Job Name: MILLWOOD 2-ELEV 1

RO

Project #:

11.875" 2-Ply - PASSED Level: Ground Floor



Member Infor	mation			Unfactored Reactions UNPATTERNED Ib (Uplift)						
Туре:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind		
Plies:	2	Design Method:	LSD	1	641	269	0	0		
Moisture Conditio	n: Dry	Building Code:	NBCC 2010 / OBC 2012	2	539	202	0	0		
Deflection LL:	360	Load Sharing:	No							
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal	Vibration:	Not Checked							
General Load										
Floor Live:	40 PSF		27	Bearings and	d Factore	d Reactions				
Dead:	15 PSF			Bearing Leng	gth C	ap. React D/L lb	Total Ld. Case	Ld. Comb.		
				1 - Steel 5.250	o" 3	8% 337 / 961	1298 L	1.25D+1.5L		
				2 - SPF 1.875	5" 4	0% 253 / 809	1061 L	1.25D+1.5L		

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	3724 ft-lb	8' 3/16"	9020 ft-lb	0.413 (41%)	1.25D+1.5L	L
Unbraced	3724 ft-lb	8' 3/16"	3737 ft-lb	0.997 (100%)	1.25D+1.5L	L
Shear	1052 lb	15'2 3/8"	3400 lb	0.309 (31%)	1.25D+1.5L	L
Perm Defl in.	0.062 (L/2891)	7'10 3/8"	0.494 (L/360)	0.120 (12%)	D	Uniform
LL Defl inch	0.164 (L/1083)	7'10 3/8"	0.494 (L/360)	0.330 (33%)	L	L
TL Defl inch	0.226 (L/788)	7'10 3/8"	0.741 (L/240)	0.300 (30%)	D+L	L

T.L. WISE 100083566 100083566 WCE OF ON

September 13, 2018

- Design Notes 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 3'9" o.c.

5 Bottom flange braced at bearings.

O DOMONI	nange braced at bearings	o.							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 0-5-4	(Span)0-3-12	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 0-5-4	(Span)0-8-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	0-2-10		Тор	1 lb	3 lb	0 lb	0 lb	J7
4	Point	0-2-10		Тор	1 lb	4 lb	0 lb	0 lb	J7
5	Point	0-2-10		Тор	1 lb	3 lb	0 lb	0 lb	J7
6	Point	0-2-10		Тор	1 lb	0 lb	0 lb	0 lb	Wall Self Weight
7	Point	0-2-10		Тор	21 lb	56 lb	0 lb	0 lb	J7
Continued or	n page 2								

Notes

Calculated Structured Designs is responsible only of the structural adequacy of title component based on the design criteria and loadings shown. It is the responsibility of the customer endor the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 Uplist not to be treated with fire retardant or corros

Handling & Installation

Handling & Installation

1. Joist flanges must not be cut or drilled

2. Refer to latest copy of the IJoist product information details for framing details, sufferer tables, web note chart. bridging details, multi-ply fastening details and handling/erection details

3. Damaged Joists must not be used

4. Design assumes top flange to be laterally restrained by attached shealthing or as specified in engineering notes.

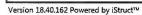
Provide lateral support at bearing points to avoid lateral displacement and rotation
 (Neb stiffeness for point load as shown Minimum point load bearing length>= 3.5 inches
 (Por flat roots provide READ ALL NOTES OF Portions)

This design is v

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Manufacturer Info

Nascor by Kott







GREENPARK

Project: Address: Date:

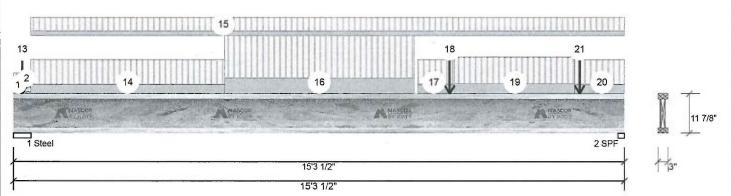
9/7/2018

Page 2 of 2

RO Designer: Job Name: MILLWOOD 2-ELEV 1

Project #:

11.875" 2-Ply - PASSED Level: Ground Floor



Continued 1	from page 1								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
8	Point	0-2-10		Тор	22 lb	59 lb	0 lb	0 lb	J7
9	Point	0-2-10		Тор	2 lb	5 lb	0 lb	0 lb	J7
10	Point	0-2-10		Тор	20 lb	0 lb	0 lb	0 lb	Wall Self Weight
11	Point	0-2-10		Тор	9 lb	25 lb	0 lb	0 lb	J7
12	Point	0-2-10		Тор	10 lb	26 lb	0 lb	0 lb	J7
13	Point	0-2-10		Тор	9 lb	0 lb	0 lb	0 lb	Wall Self Weight
14	Tie-In	0-5-4 to 5-3-10	(Span)1-7-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
15	Tie-In	0-5-4 to 15-3-8	(Span) 0-10-12	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
16	Tie-In	5-3-10 to 10-0-8	(Span)2-9-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
17	Tie-In	10-1-10 to 11-0-10	(Span)1-7-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
18	Point	10-11-2		Far Face	18 lb	49 lb	0 lb	0 lb	F9
19	Tie-In	11-0-10 to 14-0-10	(Span)1-9-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
20	Tie-In	14-0-10 to 15-3-8	(Span)1-7-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
21	Point	14-2-2		Far Face	25 lb	66 lb	0 lb	0 lb	F9

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the Intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 Uplist not to be treated with fire retardant or corrosi

chemicals

Handling & Installation

Handling & Installation

1. IJoist flanges must not be cut or drilled

2. Refer to latest copy of the IJoist product information details for framing details, stifferer tables, web hole chart, bridging details, multiply frastening details and handlingferection details

3. Damaged IJoists must not be used

4. Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web differens for point load as shown Milnimum point foad bearing length>= 3.5 inches
 For flat roofs provide proper drainage to prevent ponding

This design is valid until 7/10/2021

Manufacturer Info Nascor by Kott



Client:

Project: Address:

GREENPARK

9/7/2018 Date:

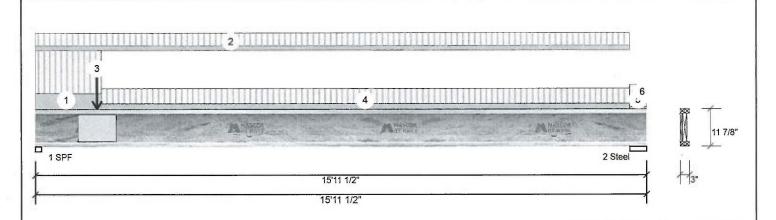
RO Designer:

Job Name: MILLWOOD 2-ELEV 1

Project #:

2-Ply - PASSED F12-B 11.875"

Level: Ground Floor



Member Infor	rmation			Unfactore	d React	ions UNPAT	TERNED I	(Uplift)	
Type:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Sr.	now	Wind
Plies:	2	Design Method:	LSD	1	712	267		0	0
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	376	141		0	0
Deflection LL:	360	Load Sharing:	No						
Deflection TL:	240	Deck:	Not Checked						
importance:	Normal	Vibration:	Not Checked						
General Load									
Floor Live:	40 PSF			Bearings a	and Fact	ored Reacti	ons		
Dead:	15 PSF	(4)		Bearing L	ength	Cap. React	D/L lb Tot	al Ld. Case	Ld. Comb.
				1 - SPF 1.	.875"	53% 334	/ 1068 140	02 L	1.25D+1.5L
				2 - Steel 5.	.250"	22% 176	6 / 563 73	9 L	1.25D+1.5L

Analysis Results

Actual	Location	Allowed	Capacity	Comb.	Case
3112 ft-lb	6'11 1/2"	9020 ft-lb	0.345 (34%)	1.25D+1.5L	L
3112 ft-lb	6'11 1/2"	3135 ft-lb	0.993 (99%)	1.25D+1.5L	L
1386 lb	1 1/8"	3400 lb	0.408 (41%)	1.25D+1.5L	L
0.057 (L/3233)	7'6 13/16"	0.516 (L/360)	0.110 (11%)	D	Uniform
0.153 (L/1213)	7'6 13/16"	0.516 (L/360)	0.300 (30%)	L	L
0.211 (L/882)	7'6 13/16"	0.774 (L/240)	0.270 (27%)	D+L	L
	3112 ft-lb 3112 ft-lb 1386 lb 0.057 (L/3233) 0.153 (L/1213)	3112 ft-lb 6'11 1/2" 3112 ft-lb 6'11 1/2" 1386 lb 1 1/8" 0.057 (L/3233) 7'6 13/16" 0.153 (L/1213) 7'6 13/16"	3112 ft-lb 6'11 1/2" 9020 ft-lb 3112 ft-lb 6'11 1/2" 3135 ft-lb 1386 lb 1 1/8" 3400 lb 0.057 (L/3233) 7'6 13/16" 0.516 (L/360) 0.153 (L/1213) 7'6 13/16" 0.516 (L/360)	3112 ft-lb 6'11 1/2" 9020 ft-lb 0.345 (34%) 3112 ft-lb 6'11 1/2" 3135 ft-lb 0.993 (99%) 1386 lb 1 1/8" 3400 lb 0.408 (41%) 0.057 (L/3233) 7'6 13/16" 0.516 (L/360) 0.110 (11%) 0.153 (L/1213) 7'6 13/16" 0.516 (L/360) 0.300 (30%)	3112 ft-lb 6'11 1/2" 9020 ft-lb 0.345 (34%) 1.25D+1.5L 3112 ft-lb 6'11 1/2" 3135 ft-lb 0.993 (99%) 1.25D+1.5L 1386 lb 1 1/8" 3400 lb 0.408 (41%) 1.25D+1.5L 0.057 (L/3233) 7'6 13/16" 0.516 (L/360) 0.110 (11%) D 0.153 (L/1213) 7'6 13/16" 0.516 (L/360) 0.300 (30%) L



- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 4'2" o.c.



September 13, 2018

2 Porrou	nange braced at bearing	ys.						
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind Comments
1	Tie-In	0-0-0 to 1-8-14	(Span)3-3-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF
2	Tie-In	0-0-0 to 15-6-4	(Span) 0-11-12 to 0-11-12	Тор	15 PSF	40 PSF	0 PSF	0 PSF
3	Point	1-7-6		Near Face	129 lb	343 lb	0 lb	0 lb F10
4	Tie-In	1-8-14 to 15-6-4	(Span)1-1-12 to 1-1-12	Тор	15 PSF	40 PSF	0 PSF	Pass-Thru Framing Squash Block is required at all point loads over bearings
5	Tie-In	15-6-4 to 15-11-8	(Span)0-5-4	Тор	15 PSF	40 PSF	0 PSF	Refers to Multiple Member Connection
6	Tie-In	15-6-4 to 15-11-8	(Span) 0-10-12	Тор	15 PSF	40 PSF	0 PSF	Details for ply to ply nailing or bolting requirements

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 Usolst not to be treated with fire retardant or corrosive

Handling & Installation

- landfling & Installation .

 Libiat flanges must not be cut or drilled.

 Refer to latest copy of the Jubist product information defails for framing details, stiffener tables, web hole teach. bridging details, multiply fastening details and handling/erection details.

 Demaged Jubist must not be used.

 Dealing assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

This design is

5. Provide lateral support at bearing points to avoid lateral displacement and rotation
6. Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
7. For flat roofs pro Nascor by Kott

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Manufacturer Info



GREENPARK

Project: Address: Date: Designer: 9/7/2018

Job Name: MILLWOOD 2-ELEV 1

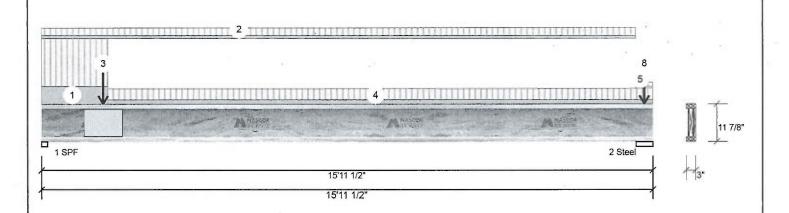
RO

Page 1 of 2

Project #:

2-Ply - PASSED 11.875"

Level: Ground Floor



Member Infor	mation			Unfactore	d Reaction	ons UNPATTERN	ED lb (Uplift)	
Type:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind
Plies:	2	Design Method:	LSD	1	649	244	0	0
Moisture Conditio	n: Dry	Building Code:	NBCC 2010 / OBC 2012	2	417	179	0	0
Deflection LL:	360	Load Sharing:	No					
Deflection TL:	240	Deck:	Not Checked					
Importance:	Normal	Vibration:	Not Checked					
General Load								
Floor Live:	40 PSF			Bearings a	nd Facto	red Reactions		
Dead:	15 PSF	- 2		Bearing Le	ength	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
				1 - SPF 1.	.875"	48% 305 / 973	1278 L	1.25D+1.5L
				2 - Steel 5.	250"	25% 223 / 625	849 L	1.25D+1.5L

Analysis Results

Alluly 313 Ites	raits						
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case	
Moment	2271 ft-lb	6'1 5/8"	9020 ft-lb	0.252 (25%)	1.25D+1.5L	L	
Unbraced	2271 ft-lb	6'1 5/8"	2271 ft-lb	1.000 (100%)	1.25D+1.5L	L	
Shear	1264 lb	1 1/8"	3400 lb	0.372 (37%)	1.25D+1.5L	L	
Perm Defl in.	0.042 (L/4432)	7'4 5/8"	0.516 (L/360)	0.080 (8%)	D	Uniform	
LL Defl inch	0.112 (L/1664)	7'4 3/4"	0.516 (L/360)	0.220 (22%)	L	L	
TL Defl inch	0.154 (L/1210)	7'4 3/4"	0.774 (L/240)	0.200 (20%)	D+L	L	
							1



- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 4'10" o.c.

5 Bottom flange braced at bearings

1_	- Bentenn manige .	orace at ecalings.								
	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
ļ	1	Tie-In	0-0-0 to 1-8-14	(Span)3-3-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	2	Tie-In	0-0-0 to 15-6-4	(Span)0-6-4 to 0-6-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
l	3	Point	1-7-6		Far Face	152 lb	404 lb	0 lb	0 lb	F10
	4	Tie-In	1-8-14 to 15-11-8	(Span)0-9-4 to 0-9-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
l	5	Tie-In	15-6-4 to 15-11-8	(Span)0-3-12	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
l	6	Point	15-8-14		Тор	32 lb	85 lb	0 lb	0 lb	J7
1										

Continued on page 2...

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 Usist not to be treated with fire retardant or con

Handling & Installation

Handling & Installation

1. IJoist flanges must not be cut or drilled

2. Refer to latest copy of the IJoist product information details for framing details, eitflener tables, web hole chart. bridging details, multi-ply fastening details and handlingferection details

3. Damaged IJoist must not be used

4. Design assumes top flange to be laterally restrained by attached shealthing or as specified in engineering notes.

- 7.

This design is

5. Provide lateral support at bearing points to avoid lateral displacement and rotation.
6. Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches.
7. For flat roofs pro

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED

Manufacturer Info

Nascor by Kott

IN THE DESIGN OF THIS COMPONENT.



September 13, 2018



Page 2 of 2



Client:

GREENPARK

Project:

Address:

Date: Designer:

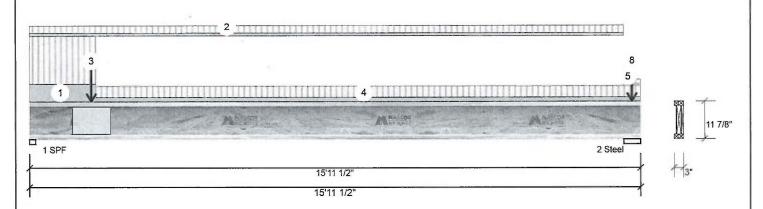
9/7/2018 RO

Job Name: MILLWOOD 2-ELEV 1

Project #:

11.875" 2-Ply - PASSED F12-C NJ

Level: Ground Floor



Continued	from	page	1
-----------	------	------	---

ł	1D	Load Type	Location Trib Width	Side	Dead	Live S	Snow \	Vind	Comments
ĺ	7	Point	15-8-14	Тор	30 lb	80 lb	0 lb	0 lb	J7
I	8	Point	15-8-14	Тор	22 lb	0 lb	0 lb	0 lb	Wall Self Weight

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component bessed on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 Usist not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

Handling & Installation

1. IJoist flanges must not be cut or drilled

2. Rafer to ialest copy of the Libist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-ply feathering details and handling/eraction details

3. Damaged Loists must not be used

4. Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeness for point load as shown Minimum point load bearing length>= 3.5 inches
 Ter flat roofs provide proper drainage to prevent ponding

This design is valid until 7/10/2021

Manufacturer Info

Nascor by Kott



GREENPARK

Project:

Address:

9/10/2018 Date:

Designer: RO

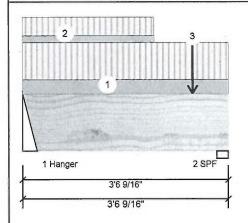
Job Name: MILLWOOD 2-ELEV 1

Project #:

Forex 2.0E-3000Fb LVL

1.750" X 11.875" - PASSED

Level: Ground Floor





1.25D+1.5L

Page 1 of 1

Member Inform	nation			Unfacto	red Reac	tions U	NPATTERN	ED lb (Uplift)	
Type:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snov	V	Wind
Plies:	1	Design Method:	LSD	1	555		218	19	0	0
Moisture Condition	: Dry	Building Code:	NBCC 2010 / OBC 2012	2	510		201		0	0
Deflection LL:	360	Load Sharing:	No							
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal	Vibration:	Not Checked							
General Load										
Floor Live:	40 PSF			Bearing	s and Fac	tored R	Reactions			
Dead:	15 PSF			Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.
				1-	3.000"	28%	272 / 833	1106	L	1.25D+1.5L
				Hanger						

2 - SPF 2.375"

40%

251 / 765

Analysis Res	sults					
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	785 ft-lb	1'9 5/16"	17130 ft-lb	0.046 (5%)	1.25D+1.5L	L
Unbraced	785 ft-lb	1'9 5/16"	13259 ft-lb	0.059 (6%)	1.25D+1.5L	L
Shear	745 lb	2'5 1/16"	5798 lb	0.129 (13%)	1.25D+1.5L	L
Perm Defl in.	0.001 (L/26669)	1'9 7/16"	0.108 (L/360)	0.010 (1%)	D	Uniform
LL Defl inch	0.004 (L/10472)	1'9 3/8"	0.108 (L/360)	0.030 (3%)	L	L
TL Defl inch	0.005 (L/7520)	1'9 3/8"	0.161 (L/240)	0.030 (3%)	D+L	L

T.L. WISE 100083566

1016 L

September 13, 2018

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.
- 4 Bottom braced at bearings.

ID	Load Type	Location T	Frib Width	Side	Dead	Live	Snow	Wind	Comments
1	Part. Uniform	0-0-0 to 3-6-9		Тор	79 PLF	210 PLF	0 PLF	0 PLF	
2	Part. Uniform	0-0-0 to 2-3-3		Near Face	39 PLF	103 PLF	0 PLF	0 PLF	
3	Point	2-11-3		Near Face	33 lb	87 lb	0 lb	0 lb	J2
	Self Weight				5 PLF				ru Framing S

Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

Calculated Structured Designs is responsible only of the structural safequecy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or col

Handling & Installation

- Handling & Installation

 1. LVL beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, mutil-tyl fastening details, beam strength values, and code approvals

 3. Damagad Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

Manufacturer Info Forex APA: PR-L318

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT. This design is ve



GREENPARK

Project: Address:

Date:

9/7/2018

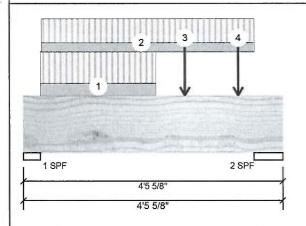
Designer: Job Name: MILLWOOD 2-ELEV 1

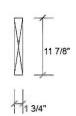
Project #:

Forex 2.0E-3000Fb LVL F4-A

1.750" X 11.875" - PASSED

Level: Ground Floor





Page 1 of 1

Member Infor	mation			Unfactor	ed React	ions U	NPATTERNI	ED lb ((Uplift)	
Туре:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snov	W	Wind
Plies:	1	Design Method:	LSD	1	1024		395		0	0
Moisture Conditio	n: Dry	Building Code:	NBCC 2010 / OBC 2012	2	1099		424		0	0
Deflection LL:	360	Load Sharing:	No							
Deflection TL:	240	Deck:	Not Checked	1						
Importance:	Normal	Vibration:	Not Checked							
General Load										
Floor Live:	40 PSF			Bearings	and Fac	tored I	Reactions			
Dead:	15 PSF			Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
		77		1-SPF	3.500"	54%	493 / 1536	2029	L	1.25D+1.5L
				2-SPF	5.875"	34%	529 / 1649	2178	L	1.25D+1.5L

Analysis Results

	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
	Moment	2002 ft-lb	2'1 9/16"	17130 ft-lb	0.117 (12%)	1.25D+1.5L	L
	Unbraced	2002 ft-lb	2'1 9/16"	11720 ft-lb	0.171 (17%)	1.25D+1.5L	L
	Shear	2314 lb	3' 5/8"	5798 lb	0.399 (40%)	1.25D+1.5L	L
	Perm Defl in.	0.004 (L/10769)	2'1 11/16"	0.127 (L/360)	0.030 (3%)	D	Uniform
	LL Defl inch	0.011 (L/4138)	2'1 11/16"	0.127 (L/360)	0.090 (9%)	L	L
3	TL Defl inch	0.015 (L/2989)	2'1 11/16"	0.191 (L/240)	0.080 (8%)	D+L	L



September 13, 2018

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Top braced at bearings.
- 3 Bottom braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Part. Uniform	0-3-8 to 2-3-8		Near Face	120 PLF	319 PLF	0 PLF	0 PLF	
2	Part. Uniform	0-3-12 to 3-11-12		Тор	90 PLF	240 PLF	0 PLF	0 PLF	
3	Point	2-9-8		Near Face	115 lb	305 lb	0 lb	0 lb	J7
4	Point	3-8-8		Near Face	112 lb	300 lb	0 lb	0 lb	J7
	Self Weight				5 PLF			Pass-Th	ru Framing

g Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or con

chemicals

Handling & Installation

- andling & Installation

 LVL beams must not be cut or drilled

 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 Demaged Beams must not be used

 Design assumes top edge is laterally restrained

 Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

This design is

Manufacturer Info Forex APA: PR-L318

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Kott Lumber Company 14 Anderson Blvd, Ontario Canada K2H7V1 905-642-4400





GREENPARK

Project: Address:

9/7/2018

Designer:

Page 1 of 1

Job Name: MILLWOOD 2-ELEV 1

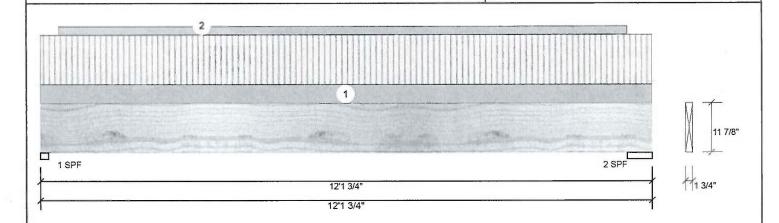
Project #:

Date:

Forex 2.0E-3000Fb LVL

1.750" X 11.875" - PASSED

Level: Ground Floor



Member Infor	lember Information					Unfactored Reactions UNPATTERNED lb (Uplift)				
Туре:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind		
Plies:	1	Design Method:	LSD	1	73	67	0	0		
Moisture Conditio	n: Dry	Building Code:	NBCC 2010 / OBC 2012	2	77	70	0	0		
Deflection LL:	360	Load Sharing:	No							
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal	Vibration:	Not Checked							
General Load										
Floor Live:	40 PSF			Bearings a	and Facto	ored Reactions				
Dead:	15 PSF			Bearing L	ength	Cap. React D/L lb	Total Ld. Case	Ld. Comb.		
11.50				1-SPF 1	.875"	10% 83 / 109	192 L	1.25D+1.5L		

2 - SPF 5.875"

3%

88 / 115

Ana	lv	sis	Re	SII	Its
Alla	IV.	212	1/5	эu	113

	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
	Moment	553 ft-lb	5'10 7/8"	17130 ft-lb	0.032 (3%)	1.25D+1.5L	L
	Unbraced	553 ft-lb	5'10 7/8"	3868 ft-lb	0.143 (14%)	1.25D+1.5L	L
	Shear	158 lb	1'1"	5798 lb	0.027 (3%)	1.25D+1.5L	L
	Perm Defl in.	0.011 (L/13107)	5'10 7/8"	0.388 (L/360)	0.030 (3%)	D	Uniform
	LL Defl inch	0.012 (L/12101)	5'10 7/8"	0.388 (L/360)	0.030 (3%)	L	L
	TL Defl inch	0.022 (L/6292)	5'10 7/8"	0.581 (L/240)	0.040 (4%)	D+L	L
Г							



1.25D+1.5L

203 L

September 13, 2018

Design Notes	
1 Girders are designed to be supported on the bottom e	edge only.

2 Top braced at bearings. 3 Bottom braced at bearings

	O DOLLOTTI DI	acca at boarings.								
Γ	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
	1	Tie-In	0-0-0 to 12-1-12	(Span)0-7-6	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	2	Part. Uniform	0-4-6 to 11-7-15		Тор	2 PLF	0 PLF	0 PLF	0 PLF	
		Self Weight				5 PLF				
1										

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or cor

Handling & Installation

landling & Installation
LVL beams must not be cut or drilled
Refer to manufacturer's product information regarding installation requirements, multi-pit fastening details, beam strength values, and code approvals
Damaged Beams must not be used
Design essumes top edge is laterally restrelned
Frovide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

This design is

Manufacturer Info Forex APA: PR-L318

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Kott Lumber Company 14 Anderson Blvd, Ontario 905-642-4400





Client:

GREENPARK

Project:

Date: 9/7/2018

Designer:

Job Name: MILLWOOD 2-ELEV 1

Unfactored Reactions UNPATTERNED Ib (Uplift)

Project #:

Forex 2.0E-3000Fb LVL F5-B

1.750" X 11.875" - PASSED

Level: Ground Floor

	a 5.		
		2	11 7
1 SPF	12'2 3/4"	2 SPF	1 3/4"
	12'2 3/4"		11.07

riciniber zinoi	macion			office feet feet of the feet o							
Type:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind			
Plies:	1	Design Method:	LSD	1	67	53	0	0			
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	95	66	0	0			
Deflection LL:	360	Load Sharing:	No								
Deflection TL:	240	Deck:	Not Checked								
Importance:	Normal	Vibration:	Not Checked								
General Load											
Floor Live:	40 PSF			Bearings a	and Fact	ored Reactions					
Dead:	15 PSF			Bearing L	ength	Cap. React D/L lb	Total Ld. Case	Ld. Comb.			
				1-SPF 1	.875"	8% 67 / 101	168 L	1.25D+1.5L			
		1		2-SPF 6	.875"	3% 82 / 142	224 L	1.25D+1.5L			

Analysis Results

Member Information

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	480 ft-lb	5'10 7/8"	17130 ft-lb	0.028 (3%)	1.25D+1.5L	L
Unbraced	480 ft-lb	5'10 7/8"	3868 ft-lb	0.124 (12%)	1.25D+1.5L	L
Shear	137 lb	1'1"	5798 lb	0.024 (2%)	1.25D+1.5L	L
Perm Defl in.	0.008 (L/16496)	5'10 7/8"	0.388 (L/360)	0.020 (2%)	D	Uniform
LL Defl inch	0.011 (L/13065)	5'10 7/8"	0.388 (L/360)	0.030 (3%)	L	L
TL Defl inch	0.019 (L/7290)	5'10 7/8"	0.581 (L/240)	0.030 (3%)	D+L	L

T.L. WISE 100083566 100083566 VCE OF ON

September 13, 2018

Design Notes	
Girders are designed to be supported on the bottom e	dae onl

- 2 Top braced at bearings.
- 3 Bottom braced at bearings

O DOLLOIN	bracea at bearings.								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 12-2-12	(Span)0-6-14	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	11-9-6 to 12-2-12	(Span)2-6-2	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	Self Weight				5 PLF				

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

and ling & Installation.

LVL beams must not be cut or drilled.
Refer to manufacturer's product information regarding installation requirements, multi-ply testiening details, beam strength values, and code approvals.

Damaged Beams must not be used.
Design assumes top edge is leterally restrained.
Provide lateral support at bearing points to avoid lateral displacement and rotation.

For flat roofs provide proper drainage to prevent ponding

This design is

Manufacturer Info Forex APA: PR-L318

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



GREENPARK

Project:

9/7/2018 Date:

Designer:

Job Name: MILLWOOD 2-ELEV 1

Project #:

11.875" NJ 2-Ply - PASSED F9-A

Level: Ground Floor





11 7/8

1.25D+1.5L

1.25D+1.5L

Page 1 of 1

lember Information				Unfactored Reactions UNPATTERNED Ib (Uplift)					
Type:	Girder		Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind
Plies:	2		Design Method:	LSD	1	48	18	0	0
Moisture Condition	on: Dry		Building Code:	NBCC 2010 / OBC 2012	2	49	18	0	0
Deflection LL:	360		Load Sharing:	No					
Deflection TL:	240		Deck:	Not Checked					
Importance:	Normal		Vibration:	Not Checked					
General Load									
Floor Live:	40 PSF				Bearings a	and Facto	ored Reactions		
Dead:	15 PSF				Bearing L	ength.	Cap. React D/L lb	Total Ld. Case	Ld. Comb.

1-SPF

2 -Hanger 1.875"

2 000'

4%

4%

23/72

23 / 73

Analysis Results										
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case	_			
Moment	27 ft-lb	8 7/8"	9020 ft-lb	0.003 (0%)	1.25D+1.5L	L				
Unbraced	27 ft-lb	8 7/8"	8539 ft-lb	0.003 (0%)	1.25D+1.5L	L				
Shear	83 lb	1 1/8"	3400 lb	0.024 (2%)	1.25D+1.5L	L				
Perm Defl in	0.000 (1./999)	0	999 000 (1 /0)	0.000 (0%)						

Perm Defl in. 0.000 (L/999) 0 999.000 (L/0) 0.000 (0%) LL Defl inch 0.000 (L/999) 0 999.000 (L/0) 0.000 (0%) TL Defl inch 0.000 (L/999) 0 999.000 (L/0) 0.000 (0%) T.L. WISE 100083566 100083566 NCE OF ON

95 L

96 L

September 13, 2018

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top flange braced at bearings.
- 6 Bottom flange braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-5-14	(Span)3-3-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component bessed on the design criteria and loadings shown. It is the responsibility of the customer end/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 Uoist not to be treated with fire retardant or corrosive

Handling & Installation

Handling & Installation

1. Joist flanges must not be cut or drilled

2. Refer to latest copy of the Joist product information details for framing details, attiferent tables, web hole chart. bridging details, multi-ply testening details and handlingferection details

3. Damaged Joists must not be used

4. Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length=3.5 inches
 For flat roofs provided provided to the point of the poin

This design is

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Manufacturer Info

Nascor by Kott



GREENPARK

Project:

Address:

Date:

9/7/2018

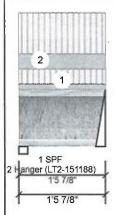
Page 1 of 1

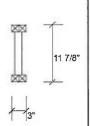
RO Designer: Job Name: MILLWOOD 2-ELEV 1

Project #:

2-Ply - PASSED NJ 11.875" F9-B

Level: Ground Floor





lember Info	ormation						Unfacto	red React	ions U	INPATTERN	ED lb	(Uplift)	
Type:	Girder		Application	n: F	loor (Residenti	al)	Brg	Live		Dead	Sno	w	Wind
Plies:	2		Design M	ethod: L	SD		1	65		24		0	0
Moisture Condi	tion: Dry		Building (Code: N	IBCC 2010 / O	BC 2012	2	66		25		0	0
Deflection LL:	360		Load Sha	ring: N	lo								
Deflection TL:	240		Deck:	٨	lot Checked								
Importance:	Normal		Vibration:	N	lot Checked								
General Load							-						
Floor Live:	40 PSF						Bearing:	s and Fact	ored	Reactions			
Dead:	15 PSF						Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
							1 - SPF	1.875"	5%	31 / 98	128	L	1.25D+1.5L
nalysis Res	ults				-		2 - Hanger	2.000"	5%	31 / 99	130	L	1.25D+1.5L
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case							
Moment	36 ft-lb	8 7/8"	9020 ft-lb	0.004 (0%)	1.25D+1.5L	L					/	FESSIO	V.
Unbraced	36 ft-lb	8 7/8"	8539 ft-lb	0.004 (0%)	1.25D+1.5L	L	1				PAR		J. D. J.
Shear	112 lb	1 1/8"	3400 lb	0.033 (3%)	1.25D+1.5L	L				-/	6		10
Perm Defl in.	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)						· f	2 /		7
LL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)						13		I.L. WIS	WALL MEER
TL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)						1-	1	000835	66
esign Note	es									/	7	enn	861

- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top flange braced at bearings.

6 Bottom flange braced at bearings.

September 13, 2018

ĺ	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
l	1	Tie-In	0-0-0 to 1-5-14	(Span)1-1-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
I	2	Tie-In	0-0-0 to 1-5-14	(Span)3-3-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Calculated Structured Designs is responsible only of the structural adequacy of this component besed on the design criteria and loadings shown, it is the responsibility of the customer endfor the contractor to ensure the component suitability of the Intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 Usist not to be treated with fire retardant or corresive

Handling & Installation

- andling & Installation

 Lioist flanges must not be cut or drilled

 Refer to latest copy of the Jubist product information details for framing details, stiffener tables, web hole chart, bridging details, multiply fastening details and handling/erection details

 Demaged Loists must not be used

 Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing leagth>= 3.5 inches
 For flat roofs provided to the stiff

This design is

ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

READ ALL NOTES ON THIS PAGE AND ON THE

Manufacturer Info

Nascor by Kott

Kott Lumber Company 14 Anderson Blvd, Ontario Canada K2H7V1 905-642-4400







Client:

GREENPARK

Project: Address:

9/10/2018 Date:

Designer:

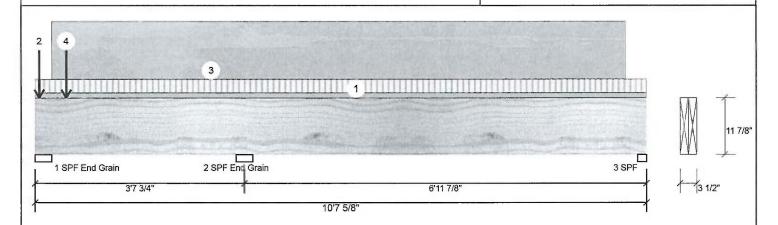
Job Name: MILLWOOD 2-ELEV 1

Project #:

Forex 2.0E-3000Fb LVL F15-A

1.750" X 11.875"

2-Ply - PASSED Level: Ground Floor



Member Inforn	nation			Unfacto	red Reac	tions U	INPATTERNI	ED lb
Туре:	Girder	Application:	Floor (Residential)	Brg	Live	!	Dead	Sno
Plies:	2	Design Method:	LSD	1	986		484	
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012	2	176		679	
Deflection LL:	360	Load Sharing:	No	3	52		259	
Deflection TL:	240	Deck:	Not Checked	"	77			
Importance:	Normal	Vibration:	Not Checked					
General Load								
Floor Live:	40 PSF			Bearing	s and Fac	tored	Reactions	
Dead:	15 PSF			Bearing	Length	Сар.	React D/L lb	Total
				1 - SPF	3.500"	24%	578 / 1504	2082

Analysis Res	ults					
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Neg Moment	-617 ft-lb	3'7 3/4"	22269 ft-lb	0.028 (3%)	1.4D	Uniform
Unbraced	-617 ft-ib	3'7 3/4"	22269 ft-lb	0.028 (3%)	1.4D	Uniform
Pos Moment	521 ft-lb	7'9"	22269 ft-lb	0.023 (2%)	1.4D	Uniform
Unbraced	521 ft-lb	7'9"	21873 ft-lb	0.024 (2%)	1.4D	Uniform
Shear	421 lb	4'7 5/8"	7537 lb	0.056 (6%)	1.4D	Uniform
Perm Defl in.	0.005 (∐17692)	7'3 7/16"	0.230 (L/360)	0.020 (2%)	D	Uniform
LL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
TL Defl inch	0.006 (L/14724)	7'3 3/8"	0.345 (L/240)	0.020 (2%)	D+L	_L

Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on full section width.

(Uplift)

Brg	Live	Dead	Snow	Wind
1	986	484	0	0
2	176	679	0	0
3	52	259	0	0

_						
Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF End Grain	3.500"	24%	578 / 1504	2082	L_	1,25D+1.5L
2 - SPF End Grain	3.500"	17%	995 / 0	995	Uniform	1.4D
3 - SPF	1.875"	13%	348 / 0	348	Uniform	1.4D



September 13, 2018

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corre

chemicals

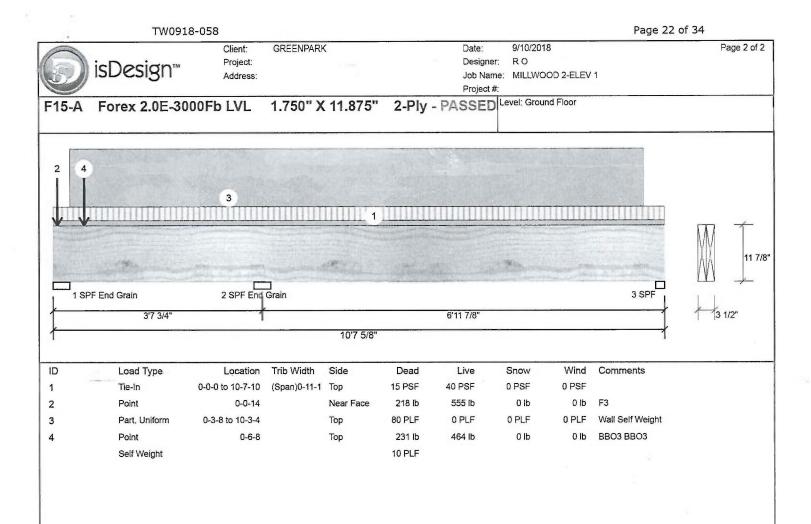
Handling & Installation

- articuling & inscallation
 LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength votues, and code
 approvals
 Dennaged Beams must not be used
 Dennaged Beams must not be used
- Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation
- For flat roofs provide proper drainage to prevent ponding

This design is

Manufacturer Info Forex APA: PR-L318

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. If is the responsibility of the customer and/or the contractor to ensure the component suitability of the Intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or con

Handling & Installation

Handling & Installation

1. LVL beams must not be cut or drilled

2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastering details, beam strength values, and code approvals

3. Damaged Beams must not be used

4. Design assumes top edge is laterally restrained

5. Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

This design is valid until 7/10/2021

Forex APA: PR-L318

Manufacturer Info

905-642-4400

S.\CUSTOMERS\GREENPARK

WINNISALE HOMESIMODELS WILLWOOD 24FLOORSIELEV 1

LSD

15

480

360

480

360

360

240

480

240

5/8"

SPF Physocol

Nailed & Glued

Gypsum 1/2"

WILLWOOD 2-ELEV 1.isf

Second Floor

Design Method Building Code NBCC 2010 / OBC 2012

Floor ahen I Live

Dead Deflection Joist

LL Span L/

Ti Snan 17

LL Cant 2L/

TL Cant 2L/

LL Span L/

TL Span L/

LL Cant 2L/

TL Cant 2L/ Decking

Deck

Thickness

Fastener

Vibration

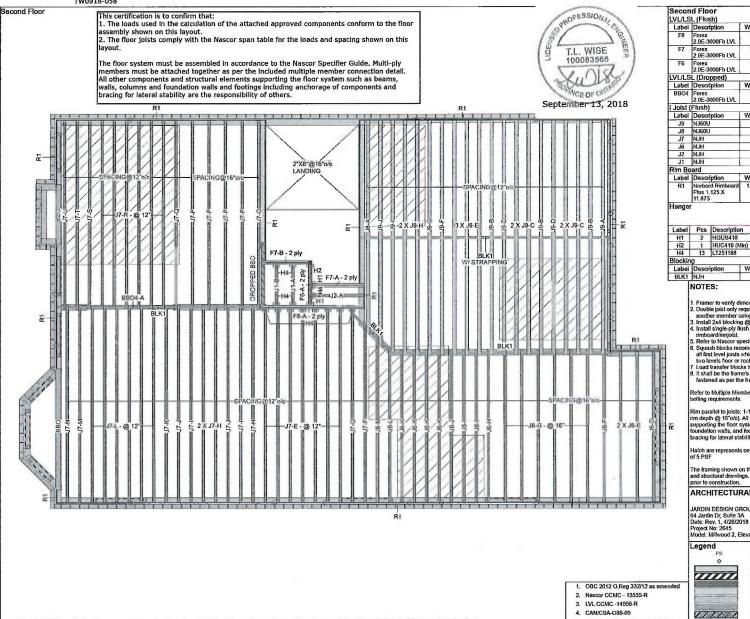
Ceiling:

Deflection Girder

Joh Path

6 104

Version 18.40.162 Powered by iStruct**



This layout is to be used as an installation guide only. It is meant to be used in conjunction with the architectural and structural drawings, not to replace them

	d Floor L (Flush)									NASCO
	Description	W	idth	Dep	th (Qty	Plies	Pcs	Length	MAJCW
F8	Forex 2.0E-3000Ft		1.75	11.8	75	1	2	2	10-0-0	Layout Name
F7	Forex 2.0E-3000Ft		1.75	11.8	75	2	2	4	6-0-0	MILLWOOD 2-ELEV 1
F6	Forex 2.0E-3000Ft		1.75	11.8	75	1	2	2	4-0-0	Design Method
LVL/LS	L (Dropped)					0.00			Description
	Description		fidth	Dep	th C	Ωty	Plies	Pcs	Length	
BBO4	Forex 2.0E-3000Ft		1.75	9	.5	1	2	2	8-0-0	Created June 25, 2018
I Joist (Flush)									Builder
Label	Description	W	idth	Dep	th (Qty	Plies	Pcs	Length	GREENPARK
19	NJ60U		3.5	11.8	75			19	20-0-0	
J8	NJ60U		3.5	11.8	75			1	18-0-0	Sales Rep
J7	NJH		2.5	11.8	75			40	16-0-0	RM
J6	HLM		2.5	11,8	75			20	14-0-D	Designer
J2	HLM		2.5	11.8	75			1	6-0-0	RO
J1	NJH		2.5	11.8	75			2	4-0-0	Shipping
Rim Bo	ard	- 33								Project
Label	Description	W	fidth	Dep	th (2ty	Plies	Pcs	Length	
R1	Norbord Rim		.125	11.8	75			14	12	Builder's Project
	Plus 1.125 X 11.875						9			Kott Lumber Company 14 Anderson Blvd
Hangei						Be	am/Girder		pported ember	Stouffville, Onlario Canada
Label	Pcs Des	cription	S	kew	Slope	fa	steners	fa	steners	K2H7V1
										DOE 642 1100

46 16d

4 10dx1 1/2 2 10dx1 1/2

14 164

 Width
 Depth
 Qty
 Plies
 Pcs
 Length

 2.5
 11.875
 LmFt
 Varies
 39-0-0

- Framer to venly dimensions on the architectural drawings.
 Double joist only require filter/backer by when supporting another member using a face mounted hanger.
 Install 2x4 blocking @ 24*0'c under parallel non-load bearing walls.
 Install style-tyly filtsh window header along inside face of
- rimboard/rimjoist. Refer to Nascor specifier guide for installation works.
- Squash blocks recommended to be installed at end bearing on all first level joists which support loading from above exceeding two levels floor or roof.
- Load transfer blocks to be installed under all point loads
- . It shall be the frame's responsibility that floor joists and beams are fastened as per the hanger manufacturer's standards.

Refer to Multiple Member Connection Detail to ply to ply nailing or

Rim parallel to joists: 1-1/8" rimboard with 2"x4" block (1/16" longer than rim depth @ 16"or's). All other components and structural elements supporting the floor system such as beams, walls, columns, and foundation walls, and footings including anochorage of components and racing for lateral stability are the responsibility of Others

Hatch are represents ceramic filed floor with an additional dead load of 5 PSF

The framing shown on this layout may deviate from the architectural and structural drawings. Project Engineer to review and apporve the deviation prior to construction.

ARCHITECTURAL DRAWINGS:

JARDIN DESIGN GROUP INC 64 Jardin Dr, Suite 3A Date: Rev. 1, 4/26/2018 Project No: 2645 Model, Mitwood 2, Elevation 1

5. CCMC -12787-R APA PR-L310(C)

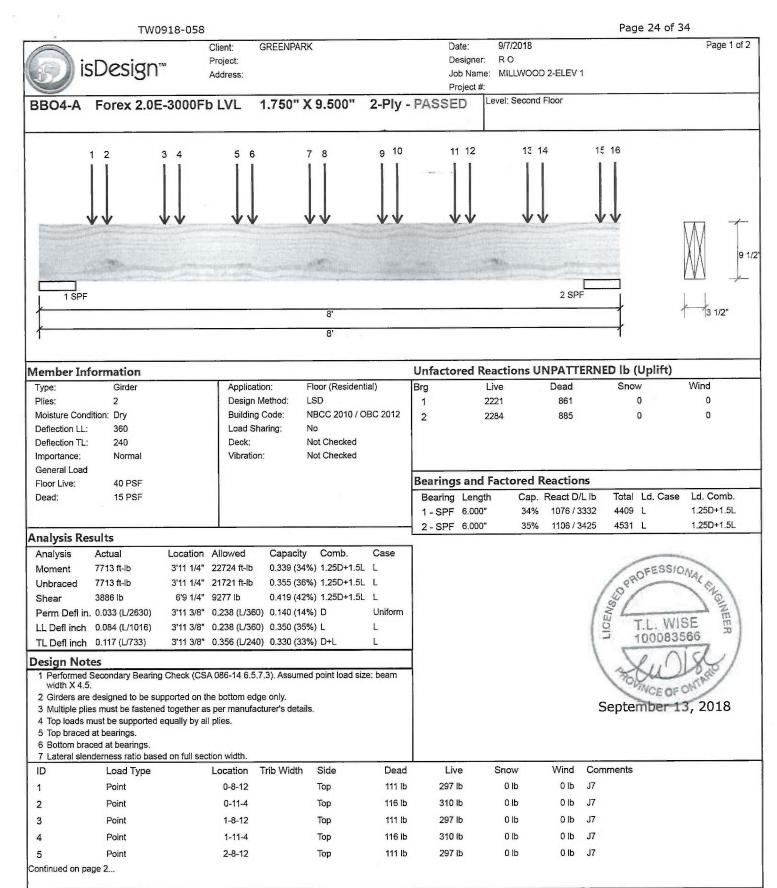
Point Load Support 0

Load from Above Wall Opening

Norbord Rimboard Plus 1,125 X 11,875 NJ 11.875

NJ60U 11.875 NJH 11.875

Forex 2.0E-3000Fb LVL 1.75 X 9.5 (Dropped) Forex 2.0E-3000Fb LVL 1.75 X 11.875



Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to werify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corr

chemicals

Handling & Installation

LVL beams must not be cut or drilled
Refer to manufacturer's product information
regarding installation requirements. multi-ply
fastening details, beam strength values, and code
approvals
Damaged Beams must not be used

Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoil lateral displacement and rotation

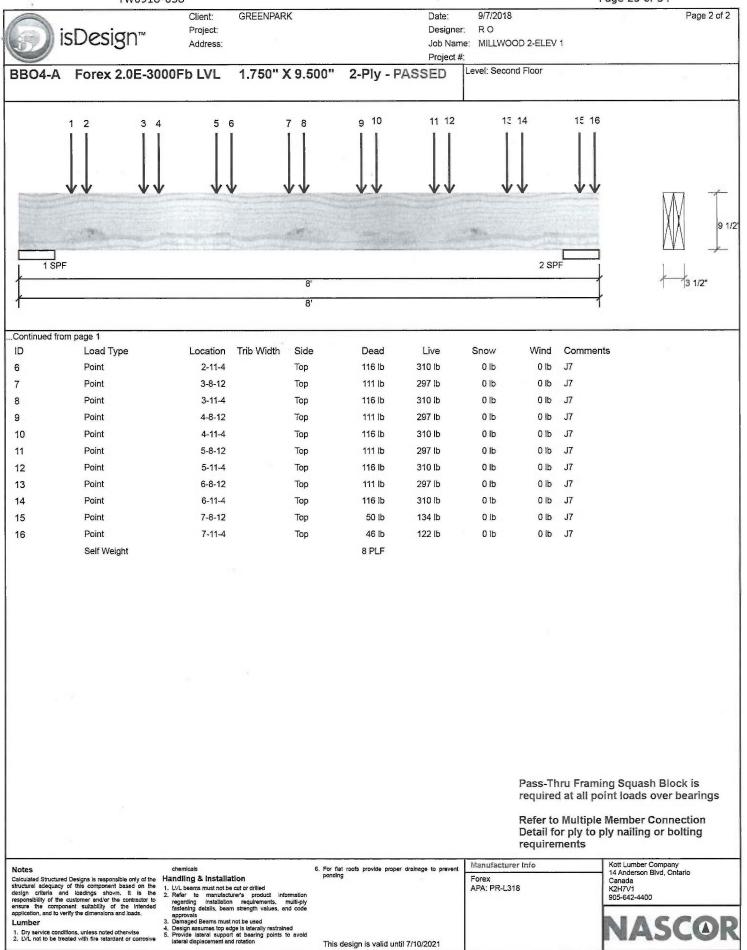
For flat roofs provide proper drainage to prevent ponding

This design is

Manufacturer Info Forex APA: PR-L318

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.







GREENPARK

Project:

Address:

Date:

9/7/2018

Page 1 of 1

Page 26 of 34

RO Designer:

Job Name: MILLWOOD 2-ELEV 1

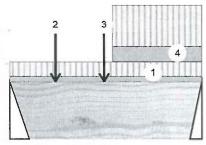
Project #:

Forex 2.0E-3000Fb LVL

1.750" X 11.875"

2-Ply - PASSED

Level: Second Floor



1 Hanger (HGUS410) 2 Hanger (HUC410 (Min)) 3'3 3/4"

11 7/8"

Member Information Unfactored Reactions UNPATTERNED Ib (Uplift) Application: Floor (Residential) Type: Brg Live Dead Snow Wind Plies: 2 Design Method: LSD 150 83 0 0 1 Moisture Condition: Dry **Building Code:** NBCC 2010 / OBC 2012 169 88 0 0 2 Deflection LL: 360 Load Sharing: No Deflection TL: Not Checked 240 Deck: Not Checked Importance: Normal Vibration: General Load **Bearings and Factored Reactions** Floor Live: 40 PSF Dead: 15 PSF Bearing Length Cap. React D/L lb Total Ld. Case Ld Comb

Analysis Results Analysis Actual Location Allowed Capacity Comb. Case 246 ft-lb Moment 1'7 1/2" 34261 ft-lb 0.007 (1%) 1.25D+1.5L L Unbraced 246 ft-lb 1'7 1/2" 34261 ft-lb 0.007 (1%) 1.25D+1.5L L Shear 246 lb 1'3 1/8" 11596 lb 0.021 (2%) 1.25D+1.5L L Perm Defl in. 0.000 (L/999) 0 999.000 (L/0) 0.000 (0%) LL Defl inch 0.000 (L/999) 0 999.000 (L/0) 0.000 (0%)

1'7 3/4" 0.145 (L/240) 0.010 (1%) D+L

104 / 225 329 1 1 25D+1 5L 4 000" 3% 1 -Hanger 2.500" 6% 110 / 253 363 L 1.25D+1.5L Hanger

Design Notes

TL Defl inch 0.001

1 Fill all hanger nailing holes.

(L/46486)

- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on full section width

BOROFESSIONAL FACE	
SSW NGIN	
T.L. WISE # 100083566	
\ Junisol	
ON NOE OF ONT ARM	
Sentember 13 2018	

September 13, 2018

/ Lateral	sienderness ratio based	on full section width.							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 3-3-12	(Span)1-4-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	0-9-7		Near Face	26 lb	70 lb	0 lb		J2
3	Point	1-7-8		Near Face	33 lb	35 lb	0 lb	PassaTh	ry-Framing Squash Block is I at all point loads over bearings
4	Tie-In	1-9-4 to 3-3-12	(Span)	Тор	15 PSF	40 PSF	0 PSF	0 PSF	at all point loads over bearings
	Self Weight		3-11-13		10 PLF				Multiple Member Connection or ply to ply nailing or bolting ments

L

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the Inlended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

LVL beams must not be cut or drilled
Refer to manufacturer's product information
regarding installation requirements, multi-ply
fastening details, beam strength values, and code
approvals

Damaged Beams must not be used

Dasign assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation

Manufacturer Info Forex APA: PR-L318

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT. This design is

Kott Lumber Company 14 Anderson Blvd, Ontario Canada K2H7V1 905-642-4400





Address:

GREENPARK

Project:

Date:

9/7/2018

Page 1 of 1

Designer: RO

Job Name: MILLWOOD 2-ELEV 1

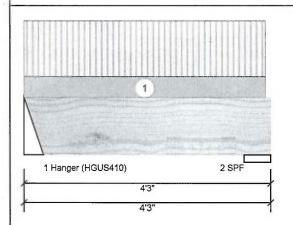
Project #:

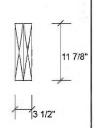
Forex 2.0E-3000Fb LVL F7-A

1.750" X 11.875"

2-Ply - PASSED

Level: Second Floor





Member Inf	ormation						Unfacto	red Reacti	ions UN	PATTERN	IED lb (Uplift)	
Туре:	Girder		Applica	tion: I	loor (Resident	ial)	Brg	Live]	Dead	Snow	Wind
Plies:	2		Design	Method:	.SD		1	35		33	0	0
Moisture Cond	ition: Dry		Building	g Code:	NBCC 2010 / C	BC 2012	2	35		34	0	0
Deflection LL:	360		Load S	•	No							
Deflection TL:	240		Deck:		Not Checked							
Importance:	Normal		Vibratio	n: I	Not Checked							
General Load							D .	I.E.	LD			
Floor Live:	40 PSF							s and Fact				
Dead:	15 PSF						Bearing	_		eact D/L lb	Total Ld. Case	Ld. Comb.
							1 - Hanger	4.000"	1%	41 / 52	93 L	1.25D+1.5L
Analysis Re	ulte						2 - SPF	5 500"	1%	43 / 53	95 L	1.25D+1.5L
		Location	Alleured	Canaditu	Carrel	Case	2-5PF	5.500	1 70	43 / 33	93 L	1.23D+1.3L
Analysis Moment	Actual 72 ft-lb			Capacity	Comb.							
	72 ft-lb		34261 ft-lb		1.25D+1.5L						ROFESSIO	MAL.
Unbraced			34261 ft-lb		1.25D+1.5L						10	181
Shear	36 lb	1'3 1/8"	11596 lb) 1.25D+1.5L	L				1	3/	SE S
Perm Defl in.				0) 0.000 (0%						- 1	H TI WIS	SE m
LL Defl inch				0) 0.000 (0%							T.L. WIS 1000835	566
TL Defl inch	0.000 (L/999)	0	999.000 (L/	0) 0.000 (0%)		4			1	T	17
Design Not	es									1	(Nul)	XY/
	er nailing holes.										12000	-CASI
	designed to be su						1				WCE OF	ON
	s must be fastene ust be supported	-		cturer's details	S.						September 1.	3, 2018
5 Top braced		equally by all	piics.									
	ed at bearings.											
	derness ratio bas	ed on full sec	tion width.									
ID	Load Type		Location	Trib Width	Side	Dead	Liv	e Sno	w Y	Wind Co	mments	
1	Tie-In	0-0-0	to 4-1-14	(Span)0-10-1	Тор	15 PSF	40 PS	F 0 P8	SF C	PSF		
	Self Weight					10 PLF						
									Pa	ss-Thru F	raming Squash E	Block is
									rec	quired at a	all point loads ov	er bearing

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the inhended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrol.

chemicals

Handling & Installation

Handling & Installation

1. LVL beams must not be cut or drilled

2. Refer to manufacturer's product information regarding installation requirements, mutil-ply fastening details, beam strength values, and code approvals

3. Damaged Beams must not be used

4. Design assumes top edge is laterally restrained

5. Provide lateral support at bearing points to avoid lateral displacement and rotation

This design is

Manufacturer Info Forex APA: PR-L318

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Kott Lumber Company 14 Anderson Blvd, Ontario Canada K2H7V1 905-642-4400

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting

requirements





GREENPARK

Project:

9/7/2018

Designer:

Job Name: MILLWOOD 2-ELEV 1

Project #:

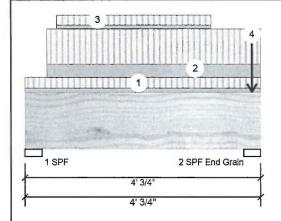
Date:

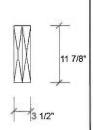
Forex 2.0E-3000Fb LVL F7-B

1.750" X 11.875"

2-Ply - PASSED

Level: Second Floor





Page 1 of 1

Member Inform	nation			Unfactor	red Reac	tions UNPATTERN	ED lb (Uplift)	
Type:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind
Plies:	2	Design Method:	LSD	1	643	260	0	0
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012	2	885	375	0	0
Deflection LL:	360	Load Sharing:	No	-				
Deflection TL:	240	Deck:	Not Checked					
Importance:	Normal	Vibration:	Not Checked					
General Load								
Floor Live:	40 PSF			Bearings	and Fac	tored Reactions		
Dead:	15 PSF			Bearing	Length	Cap. React D/L lb	Total Ld. Case	Ld. Co

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1215 ft-lb	2' 5/16"	34261 ft-lb	0.035 (4%)	1.25D+1.5L	L
Unbraced	1215 ft-lb	2' 5/16"	34261 ft-lb	0.035 (4%)	1.25D+1.5L	L
Shear	711 lb	2'10 1/8"	11596 lb	0.061 (6%)	1.25D+1.5L	L
Perm Defl in.	0.001 (L/34561)	2' 5/16"	0.120 (L/360)	0.010 (1%)	D	Uniforr
LL Defl inch	0.003 (L/13799)	2' 5/16"	0.120 (L/360)	0.030 (3%)	L	L
TL Defl inch	0.004 (L/9862)	2' 5/16"	0.180 (L/240)	0.020 (2%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on full section width.

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1-SPF	3.500"	17%	325 / 965	1289	L	1.25D+1.5L
2 - SPF End Grain	3.500"	20%	469 / 1328	1797	L	1.25D+1.5L



September 13, 2018

O Lateral	sieriderriess ratio based t	on full section width.							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 4-0-12	(Span)3-7-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Part, Uniform	0-4-8 to 4-0-12		Тор	90 PLF	240 PLF	0 PLF	0 PLF	
3	Part. Uniform	0-6-8 to 3-2-8		Near Face	25 PLF	68 PLF	0 PLF	Page_Fh	ru Framing Squash Block is
4	Point	3-11-0		Near Face	88 lb	169 lb	0 lb	required	d at all point loads over bearings
	Self Weight				10 PLF				Multiple Member Connection or ply to ply nailing or bolting nents

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and leadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

andling & Installation

LVL beams must not be cut or drilled

Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

Damaged Beams must not be used

Design assumes top edge is laterally restrained

Provide lateral support at bearing points to avoid lateral displacement and rotation

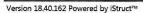
Manufacturer Info Forex APA: PR-L318

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Kott Lumber Company 14 Anderson Blvd, Ontario Canada K2H7V1 905-642-4400



This design is







Address:

GREENPARK

Project:

Date:

9/7/2018

Page 1 of 2

Designer: RO

Job Name: MILLWOOD 2-ELEV 1

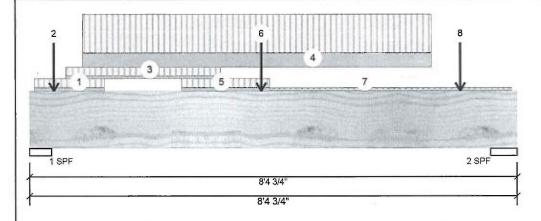
Project #:

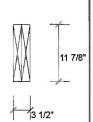
Forex 2.0E-3000Fb LVL F8-A

1.750" X 11.875"

2-Ply - PASSED

Level: Second Floor





1.25D+1.5L

Member Info	rmation			Unfactore	d Reaction	ons UNPATTERNI	ED lb (Uplift)	
Type:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind
Plies:	2	Design Method:	LSD	1	1594	649	0	0
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	1377	568	0	0
Deflection LL:	360	Load Sharing:	No					
Deflection TL:	240	Deck:	Not Checked					
Importance:	Normal	Vibration:	Not Checked					
General Load								
Floor Live:	40 PSF			Bearings a	and Facto	ored Reactions		
Dead:	15 PSF			Bearing L	ength	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
				1-SPF 4	.500"	33% 811 / 2391	3202 L	1.25D+1.5L

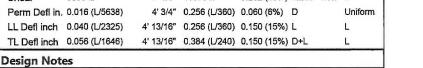
2-SPF 5.500"

23%

710 / 2066

_				
Δ	na	veic	Resu	te.

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	5966 ft-lb	4'	34261 ft-lb	0.174 (17%)	1.25D+1.5L	L
Unbraced	5966 ft-lb	4'	31511 ft-lb	0.189 (19%)	1.25D+1.5L	L
Shear	3035 lb	7' 1/8"	11596 lb	0.262 (26%)	1.25D+1.5L	L
Perm Defl in.	0.016 (L/5638)	4' 3/4"	0.256 (1/360)	0.060 (6%)	D	Uniform
LL Defl inch	0.040 (L/2325)	4' 13/16"	0.256 (L/360)	0.150 (15%)	L	L
TL Defl inch	0.056 (L/1646)	4' 13/16"	0.384 (L/240)	0.150 (15%)	D+L	L



1 Girders are designed to be supported on the bottom edge only. 2 Multiple plies must be fastened together as per manufacturer's details.

3 Top loads must be supported equally by all plies. 4 Top braced at bearings.

5 Bottom braced at bearings.

6. Lateral slenderness ratio based on full section width

(8)	PROFESSIONAL	
LICEN	T.L. WISE 100083566	1
/3	OWNCE OF ONTHE	

2776 L

September 13, 2018

O Lateral sie	idelliess fallo based	on full section width.							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-1-0 to 1-3-8	(Span)3-7-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	0-5-0		Near Face	77 lb	205 lb	0 lb	0 lb	J7
3	Part. Uniform	0-7-8 to 3-3-8		Far Face	25 PLF	68 PLF	0 PLF	0 PLF	
4	Part. Uniform	0-11-0 to 6-11-0		Near Face	115 PLF	308 PLF	0 PLF	0 PLF	
5	Tie-In	2-7-8 to 4-1-12	(Span)3-7-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
6	Point	4-0-0		Far Face	83 lb	150 lb	0 lb	0 lb	F6

Continued on page 2...

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the inlended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

Handling & Installation

1. IVL beams must not be cut or drilled

2. Refer to manufacturer's product information regarding installation requirements. multi-ply fastening details, beam strength values, and code approvals

3. Damaged Beams must not be used

4. Design assumes top edge is laterally restrained

5. Provide lateral support at bearing points to evoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info Forex APA: PR-L318

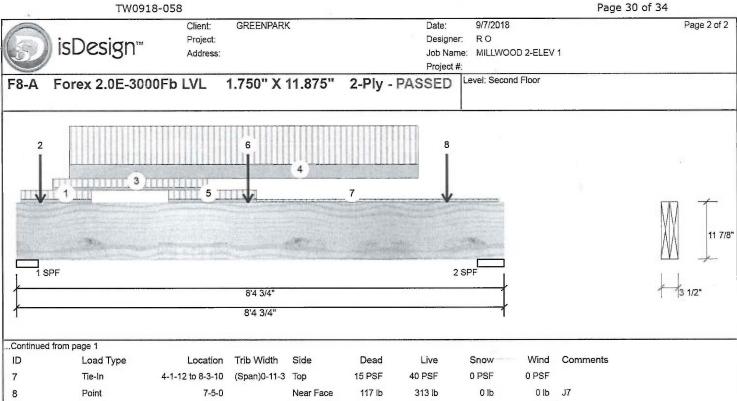
READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Kott Lumber Company 14 Anderson Blvd, Ontario Canada K2H7V1 905-642-4400



This design is





10 PLF

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Self Weight

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

and ling & Installation
LVL beams must not be cut or drilled
Refer to manufacturer's product information
regarding installation requirements, multil-pit
fastening details, beam strength values, and code
approvals
Damaged Beams must not be used
Design assumes top edge is laterally restreined
Provide lateral support at bearing points to avoid
lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

This design is valid until 7/10/2021

Manufacturer Info Forex

APA: PR-L318



GREENPARK

Project:

Date: 9/7/2018

Designer: RO

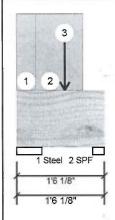
Job Name: MILLWOOD 2-ELEV 2

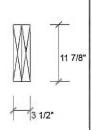
Project #:

Forex 2.0E-3000Fb LVL F15-A

1.750" X 11.875"

2-Ply - PASSED Level: Second Floor





Page 1 of 1

Member Infor	mation	Unfactored Reactions UNPATTERNED lb (Uplift)								
Type:	Girder	Application:	Floor (Residential)	Brg	Live	De	ead	Snov	N	Wind
Plies:	2	Design Method:	LSD	1	72		103		0	0
Moisture Condition	n: Dry	Building Code:	NBCC 2010 / OBC 2012	2	61		53		0	0
Deflection LL:	360	Load Sharing:	No							
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal	Vibration:	Not Checked							
General Load										
Floor Live:	40 PSF			Bearings a	nd Fact	ored Rea	ctions			
Dead:	15 PSF			Bearing Le	ength	Cap. Rea	ct D/L lb	Total	Ld. Case	Ld. Comb.
				1 - Steel 5.	250"	2%	129 / 108	237	L	1.25D+1.5L
				2 - SPF 2.	375"	3%	66 / 92	158	L	1.25D+1.5L

Analysis Results

Actual	Location	Allowed	Capacity	Comb.	Case
78 ft-lb	10"	33233 ft-lb	0.002 (0%)	1.25D+1.5L	L
78 ft-lb	10"	33233 ft-lb	0.002 (0%)	1.25D+1.5L	L
107 lb	1'4 3/8"	11248 lb	0.009 (1%)	0.9D+1.5L	L
0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
	78 ft-lb 78 ft-lb 107 lb 0.000 (L/999) 0.000 (L/999)	78 ft-lb 10" 78 ft-lb 10" 107 lb 1'4 3/8" 0.000 (L/999) 0 0.000 (L/999) 0	78 ft-lb 10" 33233 ft-lb 78 ft-lb 10" 33233 ft-lb 107 lb 1'4 3/8" 11248 lb 0.000 (L/999) 0 999.000 (L/0) 0.000 (L/999) 0 999.000 (L/0)	78 ft-lb 10" 33233 ft-lb 0.002 (0%) 78 ft-lb 10" 33233 ft-lb 0.002 (0%) 107 lb 1'4 3/8" 11248 lb 0.009 (1%) 0.000 (L/999) 0 999.000 (L/0) 0.000 (0%) 0.000 (L/999) 0 999.000 (L/0) 0.000 (0%)	78 ft-lb 10" 33233 ft-lb 0.002 (0%) 1.25D+1.5L 78 ft-lb 10" 33233 ft-lb 0.002 (0%) 1.25D+1.5L 107 lb 1'4 3/8" 11248 lb 0.009 (1%) 0.9D+1.5L 0.000 (L/999) 0 999.000 (L/0) 0.000 (0%) 0.000 (0%) 0.000 (L/999) 0 999.000 (L/0) 0.000 (0%) 0.000 (0%)

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on full section width.



September 13, 2018

O Luterar	Sichachicso ratio basca c	of fall ocodolf wider.							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Part, Uniform	0-0-0 to 0-4-0		Тор	80 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
2	Part. Uniform	0-4-0 to 1-1-12		Тор	80 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
3	Point	0-10-0		Near Face	50 lb	133 lb	0 lb	0 lb	J10
	Self Weight				10 PLF				nru Framing Squash Block is d at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design critien and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation
 LVL beams must not be cut or drilled
 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Forvoide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

This design

Manufacturer Info Forex APA: PR-L318

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT **CONTAINS SPECIFICATIONS AND CRITERIA USED** IN THE DESIGN OF THIS COMPONENT.

Kott Lumber Company 14 Anderson Blvd, Ontario Canada K2H7V1 905-642-4400





GREENPARK

Project:

9/7/2018

Designer:

Job Name: MILLWOOD 2-ELEV 3

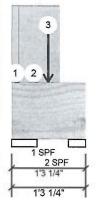
Project #:

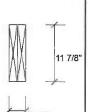
Date:

Forex 2.0E-3000Fb LVL F15-A

1.750" X 11.875"

2-Ply - PASSED Level: Second Floor





Page 1 of 1

lember Inform	nation			Unfactor	ed Reac	tions U	NPATTERN	ED lb (Uplift)	
Type:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snov	V	Wind
Plies:	2	Design Method:	LSD	1	108		101	- 0	0	0
Moisture Condition	: Dry	Building Code:	NBCC 2010 / OBC 2012	2	65		41	13	0	0
Deflection LL:	360	Load Sharing:	No	_						
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal	Vibration:	Not Checked							
General Load										
Floor Live:	40 PSF			Bearings	and Fac	tored F	Reactions			
Dead:	15 PSF			Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
				1-SPF	5.250"	3%	126 / 162	288	L	1.25D+1.5L
1	The state of the s			2-SPF	3.500"	2%	52/98	149	L	1.25D+1.5L

Analysis Results

	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case	
	Moment	58 ft-lb	7 1/2"	34261 ft-lb	0.002 (0%)	1.25D+1.5L	L	
	Unbraced	58 ft-lb	7 1/2"	34261 ft-lb	0.002 (0%)	1.25D+1.5L	L	
	Shear	190 lb	1'4 3/8"	11596 lb	0.016 (2%)	1.25D+1.5L	L	
	Perm Defl in.	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)			
	LL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)			
	TL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)			
-								-

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.



September 13, 2018

o Lateral	sienderness ratio based o	on full section width.							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Part. Uniform	0-0-0 to 0-1-8		Тор	80 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
2	Part. Uniform	0-1-8 to 0-9-12		Тор	80 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
3	Point	0-7-8		Far Face	65 lb	173 lb	0 lb	0 lb	J9
	Self Weight				10 PLF				nru Framing Squash Block is d at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corros

Handling & Installation

Handling & Installation

LVL beams must not be cut or drilled

Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

Demaged Beams must not be used

Design assumes top edge is laterally restrelned 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

This design

Manufacturer Info Forex APA: PR-L318

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Kott Lumber Company 14 Anderson Blvd, Ontario Canada K2H7V1 905-642-4400

