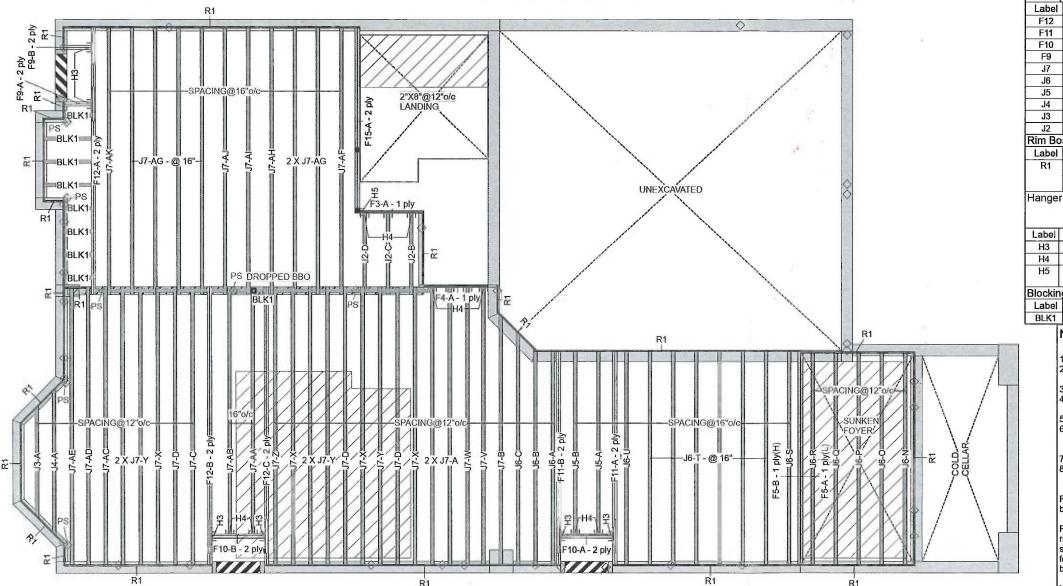
Ground Floor

Engineered floor joists shall be installed in accordance with the supplier's layout and specifications forming part of the permit drawing:

All work shall conform to the Ontario Building Code O. Reg. 332/12 as amended CITY OF BRAMPTON BUILDING DIVISION REVIEWED DEC 1-0 2018 BY MARK DERKSEN



This certification is to confirm that:

The loads used in the calculation of the attached approved components conform to the floor assembly shown on this layout.
 The floor joists comply with the Nascor span table for the loads and spacing shown on this layout.

The floor system must be assembled in accordance to the Nascor Specifier Guide. Multi-ply members must be attached together as per the included multiple member connection detail.

All other components and structural elements supporting the floor system such as beams, walls, columns and foundation walls and footings including anchorage of components and bracing for lateral stability are the responsibility of others.



1. OBC 2012 O.Reg 332/12 as amended

2. Nascor CCMC - 13535-R

3. LVL CCMC -14056-R

4. CAN/CSA-O86-09

5. CCMC -12787-R APA PR-L310(C)

LVL/LS	L (Flush)						
Label	Description	Width	Depth	Qty	Plies	Pcs	Length
F5	Forex 2.0E-3000Fb LVL	1.75	11.875			2	14-0-0
F15	Forex 2.0E-3000Fb LVL	1.75	11.875	1	2	2	12-0-0
F4	Forex 2.0E-3000Fb LVL	1.75	11.875			1	6-0-0
F3	Forex 2.0E-3000Fb LVL	1.75	11.875			1	4-0-0
I Joist (Flush)						
Label	Description	Width	Depth	Qty	Plies	Pcs	Length
F12	NJ	1.5	11.875	3	2	6	16-0-0
F11	NJ	1.5	11.875	2	2	4	14-0-0
F10	NJ	1.5	11.875	2	2	4	4-0-0
F9	NJ	1.5	11.875	2	2	4	2-0-0
J7	NJH	2.5	11.875			35	16-0-0
J6	NJH	2.5	11.875			16	14-0-0
J5	NJH	2.5	11.875			2	12-0-0
J4	NJH	2.5	11.875			1	10-0-0
J3	NJH	2.5	11.875			1	8-0-0
J2	NJH	2.5	11.875			3	6-0-0
Rim Bo	ard						
Label	Description	Width	Depth	Qty	Plies	Pcs	Length
R1	Norbord Rimboard Plus 1.125 X 11.875	1.125	11.875			12	12

Beam/Girder

 Blocking

 Label
 Description
 Width
 Depth
 Qty
 Plies
 Pcs
 Length

 BLK1
 NJH
 2.5
 11.875
 LinFt
 Varies
 29-0-0

NOTES:

Ground Floor

Framer to verify dimensions on the architectural drawings.
 Double joist only require filler/backer ply when supporting another member using a face-mounted hanger.

Install 2x4 blocking @ 24"o/c under parallel non-load bearing walls.
 Install single-ply flush window header along inside face of rimboard/rimjoist.

 Refer to Nascor specifier guide for installation works.
 Squash blocks recommended to be installed at end bearing on all first level joists which support loading from above exceeding

two levels floor or roof.

Load transfer blocks to be installed under all point loads.

It shall be the frame's responsibility that floor joists and beams are fastened as per the hanger manufacturer's standards.

Refer to Multiple Member Connection Detail to ply to ply nailing or bolling requirements.

Rim parallel to joists: 1-1/8" rimboard with 2"x4" block (1/16" longer than rim depth @ 16"o/c). All other components and structural elements supporting the floor system such as beams, walls, columns, and foundation walls, and footings including anchorage of components and bracing for lateral stability are the responsibility of Others.

Hatch are represents ceramic tiled floor with an additional dead load of 5 PSF

The framing shown on this layout may deviate from the architectural and structural drawings. Project Engineer to review and apporve the deviation prior to construction.

ARCHITECTURAL DRAWINGS:

JARDIN DESIGN GROUP INC. 64 Jardin Dr, Suite 3A Date: Rev. 1, 4/26/2018 Project No: 2645 Model: Millwood 2, Elevation 1

Legend

Point Load Support Load from Above Wall

Wall Opening Norbord Rimboard Plus 1.125 X 11.875 NJ 11.875

NJH 11.875 Forex 2.0E-3000Fb LVL 1.75 X 11.875 NASCOR

Layout Name
MILLWOOD 2-ELEV 1

Design Method
LSD

Description

Created
June 25, 2018

Builder
GREENPARK
Sales Rep
R M

Designer
R O
Shipping
Project
Builder's Project

Kott Lumber Company
14 Anderson Blvd
Stouffville, Ontario
Canada
K2H7V1
905-642-4400

Supported

Job Path
S:\CUSTOMERS\GREENPARK
\MINNISALE HOMES\MODELS
\MILLWOOD 2\FLOORS\ELEV 1
\MILLWOOD 2-ELEV 1.isl

Floor Loads 40 Live Dead 15 Deflection Joist LL Span L/ 480 360 TL Span L/ 480 LL Cant 2L/ TL Cant 2L/ 360 Deflection Girder LL Span L/ 360 TL Span L/ 240 480 LL Cant 2L/ TL Cant 2L/ 240 Decking Deck SPF Plywood **Thickness** Nailed & Glued Fastener Vibration

LET 23

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This layout is to be used as an installation guide only. It is meant to be used in conjunction with the architectural and structural drawings, not to replace them

S:\CUSTOMERS\GREENPARK MINNISALE HOMES MODELS \MILLWOOD 2\FLOORS\ELEV 1 \MILLWOOD 2-ELEV 1.isl Second Floor

Building Code NBCC 2010 / OBC

LSD

2012

40

15

480

360

480

360

360

240

480

240

SPF Plywood

Nailed & Glued

Gypsum 1/2"

14 Anderson Blvd Stouffville, Ontario Canada

K2H7V1 905-642-4400 Job Path

Design Method

Deflection Joist

LL Span L/

TL Span L/

LL Cant 2L/

TL Cant 2L/

LL Span L/

TL Span L/

LL Cant 2L/

TL Cant 2L/

Decking

Thickness

Fastener

Vibration

Deck

Deflection Girder

Floor

Loads

Live

Dead

econd Floor

This certification is to confirm that:

1. The loads used in the calculation of the attached approved components conform to the floor assembly shown on this layout.

2. The floor joists comply with the Nascor span table for the loads and spacing shown on this

The floor system must be assembled in accordance to the Nascor Specifier Guide, Multi-ply members must be attached together as per the included multiple member connection detail. All other components and structural elements supporting the floor system such as beams, walls, columns and foundation walls and footings including anchorage of components and bracing for lateral stability are the responsibility of others.



September 13, 2018

R1 2"X8"@16"o/c LANDING 6 2 X J9-C 2 X J9-C ு _2 X J9-H F7-B - 2 ply BLK1 2 X J7-H 2 X J6-E -J6-G - @ 16" -J7-E - @ 12

R1

Secon	d Floor							-vision side
LVL/LS	L (Flush)							MIACCOD
	Description	Width	Depth	Qty	Plies	Pcs	Length	NASCOR
F8	Forex 2.0E-3000Fb LVL	1.75	11.875.	1	2	2	10-0-0	
F7	Forex 2.0E-3000Fb LVL	1.75	11.875	2	2	4	6-0-0	Layout Name MILLWOOD 2-ELEV 1
F6	Forex 2.0E-3000Fb LVL	1.75	11.875	1	2	2	4-0-0	Design Method LSD
LVL/LS	L (Dropped)							
	Description	Width	Depth	Qty	Plies	Pcs	Length	Description
BBO4	Forex 2.0E-3000Fb LVL	1.75	9.5	1	2	2	8-0-0	Created June 25, 2018
I Joist (Flush)							Builder
Label	Description	Width	Depth	Qty	Plies	Pcs	Length	GREENPARK
J9	NJ60U	3.5	11.875			19	20-0-0	
J8	NJ60U	3.5	11.875			1	18-0-0	Sales Rep
J7	NJH	2.5	11.875			40	16-0-0	RM
J6	NJH	2.5	11.875			20	14-0-0	Designer
J2	NJH	2.5	11.875			1	6-0-0	RO
J1	NJH	2.5	11.875			2	4-0-0	Shipping
Rim Bo	ard		3					i
Label	Description	Width	Depth	Qty	Plies	Pcs	Length	Project
R1	Norbord Rimboard	1.125	11.875			14	12	Builder's Project
	Plus 1.125 X 11.875							Kott Lumber Company
Hanger				Be:	am/Girder	Sur	pnorted	14 Anderson Blvd Stouffville, Ontario

					Beam/Girder	Member
Label	Pcs	Description	Skew	Slope	fasteners	fasteners
H1	2	HGUS410			46 16d	16 16d
H2	1	HUC410 (Min)			14 16d	6 10d
H4	13	LT251188			4 10dx1 1/2	2 10dx1 1/2
lookin	_				•	

OCKIN	g		500				
abel	Description	Width	Depth	Qty	Plies	Pcs	Length
BLK1	NJH	2.5	11.875	LinFt		Varies	39-0-0

NOTES:

- I. Framer to verify dimensions on the architectural drawings. 2. Double joist only require filler/backer ply when supporting
- another member using a face-mounted hanger.
- Install 2x4 blocking @ 24"o/c under parallel non-load bearing walls. 4. Install single-ply flush window header along inside face of
- rimboard/rimjoist.

 5. Refer to Nascor specifier guide for installation works.
- 6. Squash blocks recommended to be installed at end bearing on all first level joists which support loading from above exceeding two levels floor or roof.
- Load transfer blocks to be installed under all point loads.
- 8. It shall be the frame's responsibility that floor joists and beams are fastened as per the hanger manufacturer's standards

Refer to Multiple Member Connection Detail to ply to ply nailing or

Rim parallel to joists: 1-1/8" rimboard with 2"x4" block (1/16" longer than rim depth @ 16"o/c). All other components and structural elements supporting the floor system such as beams, walls, columns, and oundation walls, and footings including anchorage of components and pracing for lateral stability are the responsibility of Others.

Hatch are represents ceramic tiled floor with an additional dead load

The framing shown on this layout may deviate from the architectural and structural drawings. Project Engineer to review and apporve the deviation prior to construction.

ARCHITECTURAL DRAWINGS:

JARDIN DESIGN GROUP INC. 64 Jardin Dr, Suite 3A Date: Rev. 1, 4/26/2018 Project No: 2645 Model: Millwood 2, Elevation 1

Legend

Point Load Support Load from Above

Wall Opening Norbord Rimboard Plus 1.125 X 11.875 NJ 11.875

NJ60U 11.875 NJH 11.875

Forex 2.0E-3000Fb LVL 1.75 X 9.5 (Dropped) Forex 2.0E-3000Fb LVL 1.75 X 11.875

1. OBC 2012 O.Reg 332/12 as amended

- 2. Nascor CCMC 13535-R
- 3. LVL CCMC -14056-R
- 4. CAN/CSA-086-09
- 5. CCMC -12787-R APA PR-L310(C)

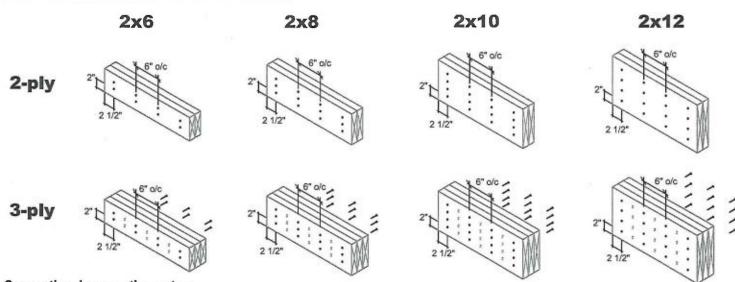
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This layout is to be used as an installation guide only. It is meant to be used in conjunction with the architectural and structural drawings, not to replace them



MULTIPLE MEMBER CONNECTIONS

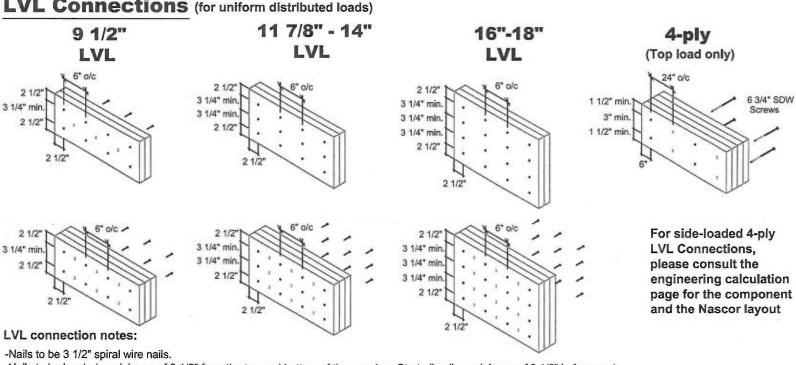
Conventional Connections (for uniform distributed loads)



Conventional connection notes:

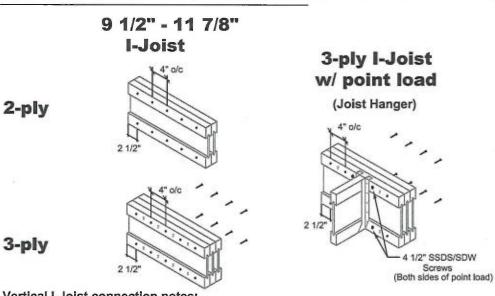
- -Nails to be 3" 10d spiral wire nails.
- -Nails to be located a minimum of 2" from the top and bottom of the member. Start all nails a minimum of 2 1/2" in from ends.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail driven from the opposite side.

LVL Connections (for uniform distributed loads)



- -Nails to be located a minimum of 2 1/2" from the top and bottom of the member. Start all nails a minimum of 2 1/2" in from ends.
- -Minimum 3 1/4" spacing between rows.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail or screw driven from the opposite side.

Vertical I-Joist Connections (for uniform distributed loads)



Vertical I-Joist connection notes:

- -Nails to be 3" spiral wire nails.
- -Nails to be located at centre of top and bottom flanges. Start all nails a minimum of 2 1/2" in from ends,
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail driven from the opposite side.

MULTI-PLY CONNECTION **DETAILS**

Date: November 30, 2016

KOTT 3228 Moodle Drive Ottawa, ON K2H 7V1 Ph: 613-838-2775

Fx: 613-838-4751

Engineering Note Page (ENP-2)

REVISION 2009-10-09

Please read all notes prior to installation of the component

DESIGN INFORMATION

This building component is certified as an individual component for the loads and conditions shown on the calculation and drawing page.

The responsibility of the undersigned engineer is <u>only</u> limited to the calculation of this building component for the loads and conditions shown on this drawing.

The responsibility of the undersigned is limited to the verification of the structural capacity of the NASCOR floor joists and LVL beams based on placement as shown on the layout. The loads applied are limited to the gravity effects of the specified loads. The structural integrity of the building and the effect of wind, uplift, seismic, lateral or other forces, calculation of adequate support and anchorage of components, as well as the dimensions and design loads used to calculate components are the responsibility of the overall building designer.

Floor joists and OSB rim board are designed to carry uniformly distributed loads only. Point loads should be transferred through the floor cavity with squash blocks. Structural elements such as walls, posts, connectors, and squash blocks are the responsibility of the overall building designer.

The undersigned engineer disclaims any responsibility for damages as a result of being furnished faulty or incorrect information, specifications and/or designs.

Installation of NASCOR joists is to be carried out in accordance with the current edition of the manufacturer's approved literature available at http://www.nascor.ca.

CODE

This building component is designed in accordance with the National Building Code of Canada, the Ontario Building Code, CCMC and Canadian Standards Association guidelines.

COMPONENT

- 1. The building component used in construction must be the same as indicated on the drawings.
- 2. The building component must be installed and assembled as per specification shown on the drawing and in accordance with the manufacturer's assembly and installation.
- 3. Members consisting of multiple plies must be connected as per the document "Multi-ply Connection Details".
- 4. Pass-thru squash block framing is required at all point loads over bearings.

HANDLING AND INSTALLATION

Do not drill any hole, cut or notch a certified building component without a written preauthorization.



MULTIPLE MEMBER CONNECTIONS

Conventional Connections (for uniform distributed loads)

2-ply







3-ply



2x6







Conventional connection notes:

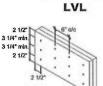
- -Nails to be 3" 10d spiral wire nails.
 -Nails to be located a minimum of 2" from the top and bottom of the member. Start all nails a minimum of 2 1/2" in from ends.

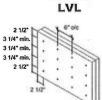
11 7/8" - 14"

- -Number of rows and spacing as per details shown, unless noted otherwise.
 "X" represents nall driven from the opposite side.

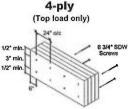
LVL Connections (for uniform distributed loads)

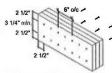
LVL

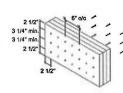


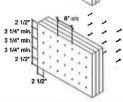


16"-18"









For side-loaded 4-ply LVL Connections, please consult the engineering calculation page for the component and the Nascor layout

LVL connection notes:

- -Nails to be 3 1/2" spiral wire nails.
 -Nails to be located a minimum of 2 1/2" from the top and bottom of the member, Start all nails a minimum of 2 1/2" in from ends.
- -Nation to be located a fill fill of the which which was all obtained the men Alliamum 3 1/4" spacing between rows.
 -Number of rows and spacing as per details shown, unless noted otherwise.
 -"X" represents nail or screw driven from the opposite side.

Vertical I-Joist Connections (for uniform distributed loads)

9 1/2" - 11 7/8" I-Joist

2-ply





3-ply I-Joist

3-ply

Vertical I-Joist connection notes:

- -Nails to be 3" spiral wire nails.
- -Nalls to be located at centre of top and bottom flanges. Start all nalls a minimum of 2 1/2" in from ends.
 -Number of rows and spacing as per details shown, unless noted otherwise.
 "X" represents nail driven from the opposite side.

MULTI-PLY CONNECTION DETAILS



4 1/2" SSDS/SDW Screws (Both sides of point load)

3228 Moodle Drive Ottawa, ON K2H 7V1 Ph: 613-838-2775

MILLWOOD 2/FLOORS/ELEV 1 WILLWOOD 2-ELEV 1.isl

Building Code NBCC 2010 / OBC

LSD

2012

40

15

480

360

490

360

360

240

480

240

SPF Plywood

Nailed & Glued

Ground Floor

Deflection Joist LL Span L/

TL Span L/

LL Cant 2L/

TL Cant 2L/

LL Span U

TL Span L/ LL Cant 2L/

TL Cant 2L/

Decking

Fastener Vibration

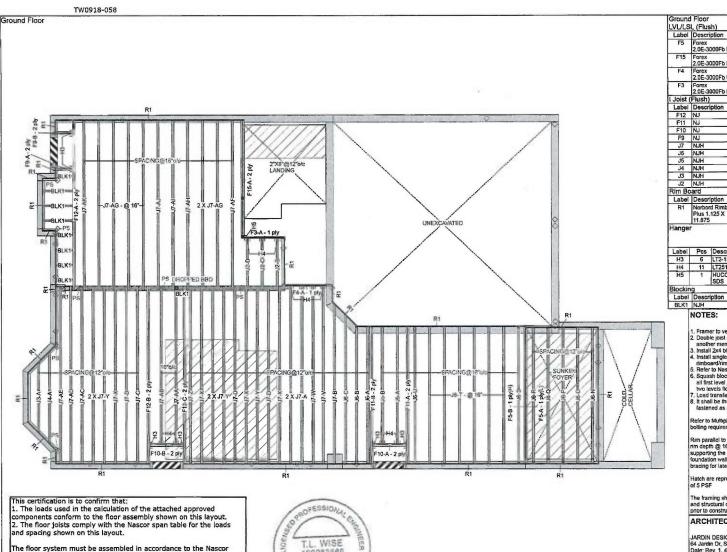
Deck Thickness

Deflection Girder

Floor

Loads

Live



The floor system must be assembled in accordance to the Nascor Specifier Guide. Multi-ply members must be attached together as per the included multiple member connection detail.

All other components and structural elements supporting the floor system such as beams, walls, columns and foundation walls and footings including anchorage of components and bracing for lateral stability are the responsibility of others.

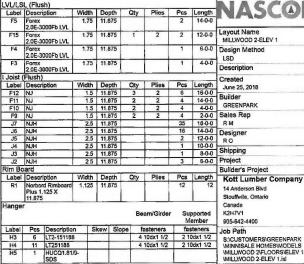
Version 18.40.162 Powered by iStruct**



September 13, 2018

This layout is to be used as an installation guide only. It is meant to be used in conjunction with the architectural and structural drawings, not to replace them

- 1. OBC 2012 O.Reg 332/12 as amende 2. Nascor CCMC - 13535-R
- 3. LVL CCMC -14056-R
- 4. CAN/CSA-086-09
- 5. CCMC -12787-R APA PR-L310(C)



NOTES:

 Width
 Depth
 City
 Piles
 Pcs
 Length
 Design Method

 2 5
 11.875
 LinFt
 Varies
 29-0-0
 Building Code

- Framer to verify dimensions on the architectural drawlings.
 Double jost only require filler/backer ply when supporting another member using a face-mounted hange.
 Install 2x4 blocking @ 24*o'c under parallel non-load bearing walls.
 Install single-ply flush window header along inside face of rimboard/irmipiosl.
 Refer to Nascor specifier grude for installation works.
 Sequash blocks recommended to be installed at end bearing and lifest lead in block with the properties of all first level joists which support loading from above exceeding
- two levels floor or roof.

 7. Load transfer blocks to be installed under all point loads. It shall be the frame's responsibility that floor joists and beams are fastened as per the hanger manufacturer's standards.

Refer to Multiple Member Connection Detail to ply to ply nailing or

Rim parallel to joists: 1-1/8" rimboard with 2"x4" block (1/16" longer than rim depth @ 16"o/c). All other components and structural element supporting the floor system such as beams, walls, columns, and foundation walls, and footings including anchorage of components and bracing for lateral stability are the responsibility of Others.

Hatch are represents ceramic tiled floor with an additional dead load of 5 PSF

The framing shown on this layout may deviate from the architectural and structural drawings. Project Engineer to review and apporve the deviation prior to construction.

ARCHITECTURAL DRAWINGS:

JARDIN DESIGN GROUP INC. 64 Jardin Dr, Suite 3A Dale: Rev. 1, 4/26/2018 Project No: 2645 Model: Millwood 2, Elevation 1

0

Legend

111111

Point Load Support Load from Above Wall Opening

Norbord Rimboard Plus 1.125 X 11.875 NJH 11.875 Forex 2.0E-3000Fb LVL 1.75 X 11.875

isDesign™

Client:

Project: Address: **GREENPARK**

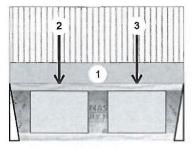
9/7/2018 Date: RO Designer:

Job Name: MILLWOOD 2-ELEV 1

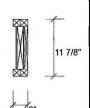
Project #

2-Ply - PASSED F10-A NJ 11.875"

Level: Ground Floor



1 Hanger (LT2-151188) 2 Hanger (LT2-151188) 3'



Mind

Page 1 of 1

Member Information

Туре: Plies: 2 Moisture Condition: Dry Deflection LL: 360 Deflection TL: 240 Importance: Normal General Load 40 PSF Floor Live: 15 PSF

Application: Design Method: **Building Code:** Load Sharing:

NBCC 2010 / OBC 2012

Floor (Residential)

Deck: Not Checked Vibration: Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

loid	Live	Dead	SHOW	VVIIIG
1	282	106	0	0
2	287	108	0	0

Bearings and Factored Reactions

Ld. Comb. Cap. React D/L lb Total Ld. Case Bearing Length 555 L 1.25D+1.5L 2.000" 20% 132 / 423 Hanger 2.000" 21% 135 / 431 566 L 1.25D+1.5L Hanger

Analysis Results

Dead:

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	401 ft-lb	1'4 1/2"	9020 ft-lb	0.044 (4%)	1.25D+1.5L	L
Unbraced	401 ft-lb	1'4 1/2"	5749 ft-lb	0.070 (7%)	1.25D+1.5L	L
Shear	558 lb	2'10 3/4"	3400 lb	0.164 (16%)	1.25D+1.5L	L
Perm Defl in.	0.001 (L/38142)	1'5 9/16"	0.093 (L/360)	0.010 (1%)	D	Uniform
LL Defl inch	0.002 (L/14284)	1'5 1/2"	0.093 (L/360)	0.030 (3%)	L	L
TL Defl inch	0.003 (L/10392)	1'5 9/16"	0.140 (L/240)	0.020 (2%)	D+L	L

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top flange braced at bearings.

2'10 3/4"	3400 lb	0.164 (16%)	1.25D+1.5L	L
1'5 9/16"	0.093 (L/360)	0.010 (1%)	D	Uniform
1'5 1/2"	0.093 (L/360)	0.030 (3%)	L	L
1'5 9/16"	0.140 (L/240)	0.020 (2%)	D+L	L
	,			



WCE OF ONTARY September 13, 2018

PROFESSIONAL PROFE

100083566

6 Bottom	flange braced at bearings	š						
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	
1	Tie-In	0-0-0 to 3-0-0	(Span)1-9-8	Тор	15 PSF	40 PSF	0 PSF	
2	Point	0-10-4		Far Face	87 lb	233 lb	0 lb	
3	Point	2-2-4		Far Face	86 lb	229 lb	0 lb	

Pass-Thru Framing Squash Block is requnted aball point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Comments

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and leadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the Intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 Upoist not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. IJoist flanges must not be cut or drilled

 2. Refer to latest copy of the IJoist product information details for framing details, stiffener tables, web hote chart. bridging details, multi-py testening details and handling/erection details

 3. Damaged IJoist must not be used

 4. Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

This design i

Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minhmum point load bearing length>= 3.5 inches
 For flat roofs ponding
 READ ALL NOTES ON 1

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Manufacturer Info

Nascor by Kott



isDesign™

GREENPARK Client:

Project: Address:

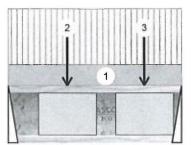
9/7/2018 Date: RO Designer:

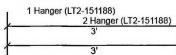
Job Name: MILLWOOD 2-ELEV 1

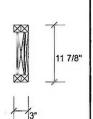
Project #

2-Ply - PASSED NJ 11.875" F10-B

Level: Ground Floor







Wind

0

0

Page 1 of 1

Member Infor	mation		
Type:	Girder	Application:	Floor (Residential)
Plies:	2	Design Method:	LSD
Moisture Conditio	n: Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		

Bearing	s and Fac	tored F
Bearing	Length	Cap.
		25%
		Bearings and Fac

Brg

1

2

l	Bearing:	s and Fac	tored l	Reactions			
	Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.
	1 - Hanger	2.000"	25%	161 / 514	675	L	1.25D+1.5L
	2 - Hanger	2.000"	29%	189 / 606	795	L	1.25D+1.5L

Unfactored Reactions UNPATTERNED Ib (Uplift)

Dead

129

152

Snow

0

0

Live

343

404

Analysis Results Location Allowed Comb. Case Analysis Actual Capacity Moment 583 ft-lb 1' 1/4" 9020 ft-lb 0.065 (6%) 1.25D+1.5L L Unbraced 583 ft-lb 1' 1/4" 5749 ft-lb 0.101 (10%) 1.25D+1.5L L Shear 788 lb 2'10 3/4" 3400 lb 0.232 (23%) 1.25D+1.5L L Perm Defl in. 0.001 1'1 5/16" 0.093 (L/360) 0.010 (1%) D Uniform (L/27610) 0.003 1'1 5/16" 0.093 (L/360) 0.030 (3%) L LL Defl inch (L/10370)0.004 (L/7538) 1'1 5/16" 0.140 (L/240) 0.030 (3%) D+L TL Defl inch

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top flange braced at bearings.

	September 1
Vind	Comments

e Rottou	i liange braceo al bealings	S						
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind
1	Tie-In	0-0-0 to 3-0-0	(Span)1-9-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF
2	Point	1-0-4		Far Face	130 lb	346 lb	0 lb	0 lb
3	Point	2-4-4		Far Face	110 lb	293 lb	0 lb	Pass Thr

J7 nru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. If it is the responsibility of the customer and/or the contractor to ensure the component suitability of the inlended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 Upost not to be treated with fire retardant or corrosive

Handling & Installation

Iandiling & Installation

. Noist flanges must not be cut or drilled

. Refer to latest copy of the Libist product information
detals for framing detals, stiffener tables, web hole
chart, bridging detals, multiply fastening detals and
handlingferection detals

. Damaged Libist must not be used

. Deslign assumes top flange to be laterally restrained
by attached sheathing or as specified in engineering
notes.

Provide lateral support at bearing points to avoid lateral displacement and rotation.
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 For that roofs
 READ ALL NOTES ON 1

READ ALL NOTES ON THIS PAGE AND ON THE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE

Manufacturer Info

Nascor by Kott

Kott Lumber Company 14 Anderson Blvd, Ontario Canada K2H7V1 905-642-4400

T.L. WISE

100083566

3, 2018





GREENPARK Client:

Project: Address:

9/7/2018 Date:

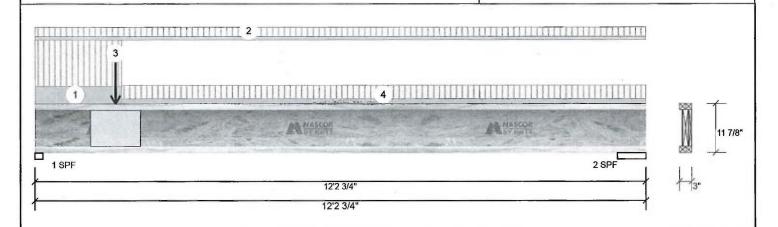
Designer: RO

Job Name: MILLWOOD 2-ELEV 1

Project #:

2-Ply - PASSED 11.875" NJ

Level: Ground Floor



Member Info	rmation		Unfactored Reactions UNPATTERNED Ib (Uplift)							
Туре:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snov	v	Wind
Plies:	2	Design Method:	LSD	1	512		192	(0	0
Moisture Conditi	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	243		91	(0	0
Deflection LL:	360	Load Sharing:	No							
Deflection TL:	240	Deck:	Not Checked	1						
Importance:	Normal	Vibration:	Not Checked							
General Load										
Floor Live:	40 PSF			Bearings	and Fac	tored	Reactions			
Dead:	15 PSF			Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
				1 - SPF	1.875"	38%	240 / 767	1008	L	1.25D+1.5L
				2-SPF	6.875"	14%	114 / 365	479	L	1.25D+1.5L

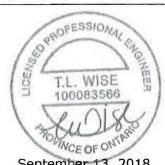
Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1602 ft-lb	4'6 5/8"	9020 ft-lb	0.178 (18%)	1.25D+1.5L	L
Unbraced	1602 ft-lb	4'6 5/8"	1617 ft-lb	0.991 (99%)	1.25D+1.5L	L
Shear	993 lb	1 1/8"	3400 lb	0.292 (29%)	1.25D+1.5L	L
Perm Defl in.	0.018 (L/7877)	5'6 5/16"	0.388 (L/360)	0.050 (5%)	D	Uniform
LL Defl inch	0.047 (L/2957)	5'6 5/16"	0.388 (L/360)	0.120 (12%)	L	L
TL Defl inch	0.065 (L/2150)	5'6 5/16"	0.581 (L/240)	0.110 (11%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 5'7" o.c.

5 Bottom flange braced at bearings.



September 13, 2018

		0-1							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-8-14	(Span)3-3-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 12-2-12	(Span)0-7-12	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	1-7-6		Far Face	108 lb	287 lb	0 lb	0 lb	F10
4	Tie-In	1-8-14 to 12-2-12	(Span)0-11-4	Тор	15 PSF	40 PSF	0 PSF		ru Framing Squash Block is

required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 Upist not to be treated with fire retardant or con

Handling & Installation

- I. IJoist flanges must not be cut or drilled
 Refer to latest copy of the IJoist product information details for framing details, shiftener tables, web hole chart, bridging details, multi-ply fastening details and handling-terection details
 Damaged tablists must not be used
- usinged uolists must not be used Design assumes top flange to be laterally restrained by attached shealthing or as specified in engineering notes.
- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 For flat roofs pro

This design is

Manufacturer Info Nascor by Kott

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



Client:

GREENPARK

Project: Address:

9/7/2018 Date:

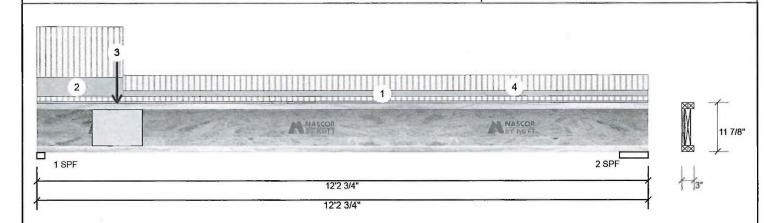
RO Designer:

Job Name: MILLWOOD 2-ELEV 1

Project #:

2-Ply - PASSED F11-B NJ 11.875"

Level: Ground Floor



Member Inf	ormation			1-4			Unfacto	red React	ions U	INPATTERN	ED lb	(Uplift)	
Туре:	Girder		Application	n: Flo	oor (Resident	ial)	Brg	Live		Dead	Sno	w	Wind
Plies:	2		Design M	ethod: LS	SD		1	471		177		0	0
Moisture Cond	lition: Dry		Building (Code: NE	BCC 2010 / O	BC 2012	2	206		77		0	0
Deflection LL:	360		Load Sha	ring: No	0								
Deflection TL:	240		Deck:	No	ot Checked		1						
Importance:	Normal		Vibration:	No	ot Checked								
General Load													
Floor Live:	40 PSF						Bearing	s and Fact	ored	Reactions			
Dead:	15 PSF						Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
							1-SPF	1.875"	35%	221 / 707	928	L	1.25D+1.5L
							2-SPF	6.875"	12%	96 / 308	405	L	1.25D+1.5L
Analysis Res	sults											_	
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case					1	SESSIO	
Moment	1410 ft-lb	4'3 1/4"	9020 ft-lb	0.156 (16%)	1.25D+1.5L	L					108	OFESSIO	MALON
Unbraced	1410 ft-lb	4'3 1/4"	1412 ft-lb	0.998 (100%)	1.25D+1.5L	L				/	Sal C		SE SEER
Shear	915 lb	1 1/8"	3400 lb	0.269 (27%)	1.25D+1.5L	L				1	可 -	TI. WIS	SE m
Perm Defl in.	0.015 (L/9004)	5'5 11/16"	0.388 (L/360)	0.040 (4%)	D	Uniform				1	i i	1000835	66 3
										1		1000000	4

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1410 ft-lb	4'3 1/4"	9020 ft-lb	0.156 (16%)	1.25D+1.5L	L.
Unbraced	1410 ft-lb	4'3 1/4"	1412 ft-lb	0.998 (100%)	1.25D+1.5L	L
Shear	915 lb	1 1/8"	3400 lb	0.269 (27%)	1.25D+1.5L	L
Perm Defl in.	0.015 (L/9004)	5'5 11/16"	0.388 (L/360)	0.040 (4%)	D	Uniform
LL Defl inch	0.041 (L/3379)	5'5 11/16"	0.388 (L/360)	0.110 (11%)	L	L
TL Defl inch	0.057 (L/2457)	5'5 11/16"	0.581 (L/240)	0.100 (10%)	D+L	L

Design Notes 1 Girders are designed to be supported on the bottom edge only.

2 Multiple plies must be fastened together as per manufacturer's details.

4 Top fla	ads must be supported ed inge must be laterally bra in flange braced at bearing	ced at a maximum of 5	i'11" o.c.							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments	
1	Tie-In	0-0-0 to 12-2-12	(Span)0-3-12	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
2	Tie-In	0-0-0 to 1-8-14	(Span)3-3-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
3	Point	1-7-6		Near Face	106 lb	282 lb	0 lb	0 lb	F10	

Top

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Pa&SSfhru Framing Squash Block is required at all point loads over bearings

Notes

4

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design critical and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the Intended application, and to verify the dimensions and loads.

Tie-In

Lumber

Dry service conditions, unless noted otherwise
 Unless not to be treated with fire retardant or corrosive

1-8-14 to 12-2-12

Handling & Installation

nannuling & Installation

1. Joist flanges must not be cut or driffed

2. Refer to latest copy of the Joist product information details for framing details, stiffener tables, web hole chart. bridging details, multi-ply fastening details and handling-ferection details

3. Damaged Loists must not be used

4. Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

(Span)

Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing langth>= 3.5 inches
 For flat roofs pro

15 PSF

40 PSF

This design is

READ ALL NOTES ON THIS PAGE AND ON THE

ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

0 PSF

Manufacturer Info

Nascor by Kott

Kott Lumber Company 14 Anderson Blvd, Ontario Canada K2H7V1

WCE OF OH!

September 13, 2018







Client:

Project: Address:

GREENPARK

9/7/2018 Date:

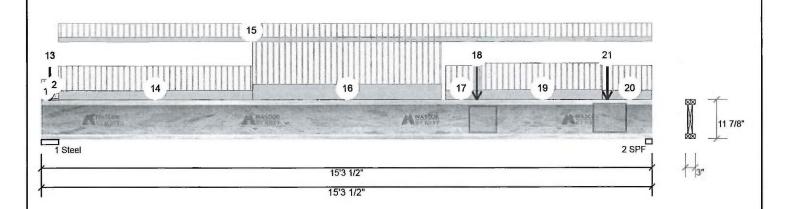
RO Designer:

Job Name: MILLWOOD 2-ELEV 1

Project #:

2-Ply - PASSED NJ 11.875"

Level: Ground Floor



Vlember Infor	mation			Unfactored Reactions UNPATTERNED Ib (Uplift)						
Type:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind		
Plies:	2	Design Method:	LSD	1	641	269	0	0		
Moisture Condition	n: Dry	Building Code:	NBCC 2010 / OBC 2012	2	539	202	0	0		
Deflection LL:	360	Load Sharing:	No							
Deflection TL:	240	Deck:	Not Checked	1						
Importance:	Normal	Vibration:	Not Checked							
General Load										
Floor Live:	40 PSF			Bearings ar	nd Factored	Reactions				
Dead:	15 PSF			Bearing Le	ngth Cap	. React D/L lb	Total Ld. Case	Ld. Comb.		
				1 - Steel 5.2	50" 389	% 337 / 961	1298 L	1.25D+1.5L		
				2 - SPF 1.8	75" 409	% 253 / 809	1061 L	1.25D+1.5L		
Ampleois Docul	4-									

Analysis Results

1							
	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
ı	Moment	3724 ft-lb	8' 3/16"	9020 ft-lb	0.413 (41%)	1.25D+1.5L	L
	Unbraced	3724 ft-lb	8' 3/16"	3737 ft-lb	0.997 (100%)	1.25D+1.5L	L
	Shear	1052 lb	15'2 3/8"	3400 lb	0.309 (31%)	1.25D+1.5L	L
	Perm Defl in.	0.062 (L/2891)	7'10 3/8"	0.494 (L/360)	0.120 (12%)	D	Uniform
	LL Defl inch	0.164 (L/1083)	7'10 3/8"	0.494 (L/360)	0.330 (33%)	L	L
	TL Defl inch	0.226 (L/788)	7'10 3/8"	0.741 (L/240)	0.300 (30%)	D+L	L
_							

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 3'9" o.c.
- 5 Bottom flange braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 0-5-4	(Span)0-3-12	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 0-5-4	(Span)0-8-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	0-2-10		Тор	1 lb	3 lb	0 lb	0 lb	J7
4	Point	0-2-10		Тор	1 lb	4 lb	0 lb	0 lb	J7
5	Point	0-2-10		Тор	1 lb	3 lb	0 lb	0 lb	J7
6	Point	0-2-10		Тор	1 lb	0 lb	0 Њ	0 lb	Wall Self Weight
7	Point	0-2-10		Тор	21 lb	56 lb	0 lb	0 lb	J7
	_								

Continued on page 2...

Notes Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 Upoist not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- Handling & Installation

 1. Julist flanges must not be cut or drifted

 2. Refer to latest copy of the IJoist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-ply f

Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 For fact roofs provide READ ALL NOTES Of MINIMUM PROPERTY AND ALL NOTES OF ALL NO

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED

Manufacturer Info

Nascor by Kott

IN THE DESIGN OF THIS COMPONENT. This design is v



September 13, 2018



Page 2 of 2



Client:

GREENPARK

Project: Address:

9/7/2018 Date:

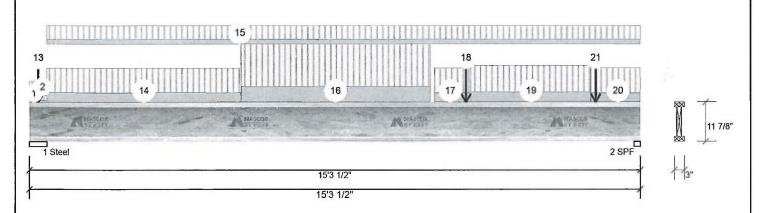
Designer:

Job Name: MILLWOOD 2-ELEV 1

Project #:

11.875" NJ 2-Ply - PASSED F12-A

Level: Ground Floor



Continued	from page 1						,			
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments	
8	Point	0-2-10		Тор	22 lb	59 lb	0 lb	0 lb	J7	
9	Point	0-2-10		Тор	2 lb	5 lb	0 lb	0 lb	J7	
10	Point	0-2-10		Тор	20 lb	0 lb	0 lb	0 lb	Wall Self Weight	
11	Point	0-2-10		Тор	9 lb	25 lb	0 lb	0 lb	J7	
12	Point	0-2-10		Тор	10 lb	26 lb	0 lb	0 lb	J7	
13	Point	0-2-10		Тор	9 lb	0 lb	0 lb	0 lb	Wall Self Weight	
14	Tie-In	0-5-4 to 5-3-10	(Span)1-7-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
15	Tie-In	0-5-4 to 15-3-8	(Span) 0-10-12	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
16	Tie-In	5-3-10 to 10-0-8	(Span)2-9-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
17	Tie-In	10-1-10 to 11-0-10	(Span)1-7-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
18	Point	10-11-2		Far Face	18 lb	49 lb	0 lb	0 lb	F9	
19	Tie-In	11-0-10 to 14-0-10	(Span)1-9-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
20	Tie-In	14-0-10 to 15-3-8	(Span)1-7-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
21	Point	14-2-2		Far Face	25 lb	66 lb	0 lb	0 lb	F9	

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Calculated Structured Designs is responsible only of the structural edequecy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- Handling & Installation

 1. IJoist flanges must not be cut or drilled

 2. Refer to latest copy of the IJoist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-ply fastening details and handling/erection details

 3. Damaged IJoist must not be used

 4. Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web sifferens for point load as shown Minimum point load bearing length>= 3.5 inches
 For flat roofs provide proper drainage to prevent ponding

This design is valid until 7/10/2021

Manufacturer Info

Nascor by Kott



GREENPARK Client:

Project: Address:

9/7/2018 Date:

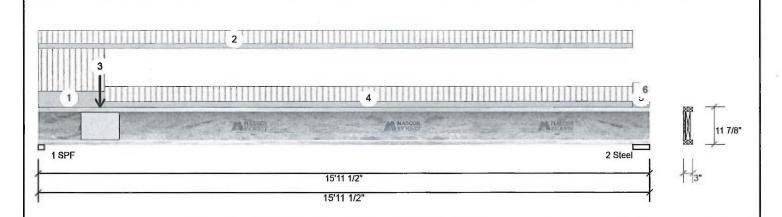
Designer: RO

Job Name: MILLWOOD 2-ELEV 1

Project #:

11.875" 2-Ply - PASSED NJ

Level: Ground Floor



Member Inform	nation			Unfactored Reactions UNPATTERNED lb (Uplift)						
Туре:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind		
Plies:	2	Design Method:	LSD	1	712	267	0	0		
Moisture Condition	: Dry	Building Code:	NBCC 2010 / OBC 2012	2	376	141	0	0		
Deflection LL:	360	Load Sharing:	No							
Deflection TL:	240	Deck:	Not Checked	1						
Importance:	Normal	Vibration:	Not Checked							
General Load										
Floor Live:	40 PSF			Bearings	and Facto	red Reactions				
Dead:	15 PSF			Bearing L	_ength	Cap. React D/L lb	Total Ld. Case	Ld. Comb.		
				1 - SPF 1	1.875"	53% 334 / 1068	1402 L	1.25D+1.5L		
				2 - Steel 5	5.250"	22% 176 / 563	739 L	1.25D+1.5L		

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	3112 ft-lb	6'11 1/2"	9020 ft-lb	0.345 (34%)	1.25D+1.5L	L
Unbraced	3112 ft-lb	6'11 1/2"	3135 ft-lb	0.993 (99%)	1.25D+1.5L	L
Shear	1386 lb	1 1/8"	3400 lb	0.408 (41%)	1.25D+1.5L	L
Perm Defl in.	0.057 (L/3233)	7'6 13/16"	0.516 (L/360)	0.110 (11%)	D	Uniform
LL Defl inch	0.153 (L/1213)	7'6 13/16"	0.516 (L/360)	0.300 (30%)	L	L
TL Defl inch	0.211 (L/882)	7'6 13/16"	0.774 (L/240)	0.270 (27%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 4'2" o.c.

5 Rottom flange braced at bearings



drige braced at bearinge								
Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
Tie-In	0-0-0 to 1-8-14	(Span)3-3-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
Tie-In	0-0-0 to 15-6-4	(Span) 0-11-12 to 0-11-12	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
Point	1-7-6		Near Face	129 lb	343 lb	0 lb	0 lb	F10
Tie-In	1-8-14 to 15-6-4	(Span)1-1-12 to 1-1-12	Тор	15 PSF	40 PSF	0 PSF	required	ru Framing Squash Block is at all point loads over bearings
Tie-In	15-6-4 to 15-11-8	(Span)0-5-4	Тор	15 PSF	40 PSF	0 PSF	Referto	Multiple Member Connection
Tie-In	15-6-4 to 15-11-8	(Span) 0-10-12	Тор	15 PSF	40 PSF	0 PSF		r ply to ply nailing or bolting
	Load Type Tie-In Tie-In Point Tie-In Tie-In	Load Type Location Tie-In 0-0-0 to 1-8-14 Tie-In 0-0-0 to 15-6-4 Point 1-7-6 Tie-In 1-8-14 to 15-6-4 Tie-In 15-6-4 to 15-11-8	Load Type Location Trib Width Tie-In 0-0-0 to 1-8-14 (Span)3-3-0 Tie-In 0-0-0 to 15-6-4 (Span) 0-11-12 to 0-11-12 to 0-11-12 Point 1-7-6 (Span)11-1-12 to 1-1-12 Tie-In 1-8-14 to 15-6-4 (Span)0-5-4 Tie-In 15-6-4 to 15-11-8 (Span)0-5-4 Tie-In 15-6-4 to 15-11-8 (Span)	Load Type Location Trib Width Side Tie-In 0-0-0 to 1-8-14 (Span)3-3-0 Top Tie-In 0-0-0 to 15-6-4 (Span) Top 0-11-12 to 0-11-12 0-11-12 to 0-11-12 Near Face Tie-In 1-8-14 to 15-6-4 (Span)1-1-12 Top to 1-1-12 Tie-In 15-6-4 to 15-11-8 (Span)0-5-4 Top Tie-In 15-6-4 to 15-11-8 (Span) Top	Load Type Location Trib Width Side Dead Tie-In 0-0-0 to 1-8-14 (Span)3-3-0 Top 15 PSF Tie-In 0-0-0 to 15-6-4 (Span) Top 15 PSF 0-11-12 to 0-11-12 0-11-12 to 0-11-12 Near Face 129 lb Tie-In 1-8-14 to 15-6-4 (Span)1-1-12 Top 15 PSF 15 PSF Tie-In 15-6-4 to 15-11-8 (Span)0-5-4 Top 15 PSF Tie-In 15-6-4 to 15-11-8 (Span) Top 15 PSF	Load Type Location Trib Width Side Dead Live Tie-In 0-0-0 to 1-8-14 (Span)3-3-0 Top 15 PSF 40 PSF Tie-In 0-0-0 to 15-6-4 (Span) Top 15 PSF 40 PSF Point 1-7-6 Near Face 129 lb 343 lb Tie-In 1-8-14 to 15-6-4 (Span)1-1-12 Top 15 PSF 40 PSF Tie-In 15-6-4 to 15-11-8 (Span)0-5-4 Top 15 PSF 40 PSF Tie-In 15-6-4 to 15-11-8 (Span) Top 15 PSF 40 PSF	Load Type Location Trib Width Side Dead Live Snow Tie-In 0-0-0 to 1-8-14 (Span)3-3-0 Top 15 PSF 40 PSF 0 PSF Tie-In 0-0-0 to 15-6-4 (Span) Top 15 PSF 40 PSF 0 PSF Point 1-7-6 Near Face 129 lb 343 lb 0 lb Tie-In 1-8-14 to 15-6-4 (Span)1-1-12 Top 15 PSF 40 PSF 0 PSF Tie-In 15-6-4 to 15-11-8 (Span)0-5-4 Top 15 PSF 40 PSF 0 PSF Tie-In 15-6-4 to 15-11-8 (Span) Top 15 PSF 40 PSF 0 PSF	Load Type Location Trib Width Side Dead Live Snow Wind Tie-In 0-0-0 to 1-8-14 (Span)3-3-0 Top 15 PSF 40 PSF 0 PSF 0 PSF Tie-In 0-0-0 to 15-6-4 (Span) Top 15 PSF 40 PSF 0 PSF 0 PSF Point 1-7-6 Near Face 129 lb 343 lb 0 lb Pass-Th Tie-In 1-8-14 to 15-6-4 (Span)1-1-12 Top 15 PSF 40 PSF 0 PSF required Tie-In 15-6-4 to 15-11-8 (Span)0-5-4 Top 15 PSF 40 PSF 0 PSF Details fo Tie-In 15-6-4 to 15-11-8 (Span) Top 15 PSF 40 PSF 0 PSF Details fo

Notes

Calcurated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the coeractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 Uses not to be treated with fire retardant or corrosive.

Handling & Installation

- andling & Installation

 Noist flanges must not be cut or drilled.
 Refer to latest copy of the Noist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-byp fastening details and handling/erection details

 Damaged Noists must not be used

 Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length= 3.5 inches
 For fiet roofs pro

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED

IN THE DESIGN OF THIS COMPONENT.

This design is

Manufacturer Info Nascor by Kott





Client: **GREENPARK**

Project: Address: Date: 9/7/2018

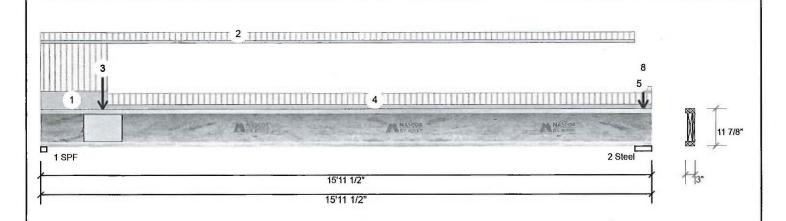
Designer: RO

Job Name: MILLWOOD 2-ELEV 1

Project #:

F12-C 11.875" 2-Ply - PASSED NJ

Level: Ground Floor



Aember Inforn	nation			Unfactor	ed React	ions U	NPATTERNI	ED lb (Uplift)	
Type:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snov	V	Wind
Plies:	2	Design Method:	LSD	1	649		244		0	0
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012	2	417		179		0	0
Deflection LL:	360	Load Sharing:	No							
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal	Vibration:	Not Checked							
General Load										
Floor Live:	40 PSF			Bearings	and Fac	tored F	Reactions			
Dead:	15 PSF			Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.
				1 - SPF	1.875"	48%	305 / 973	1278	L	1.25D+1.5L
				2 - Steel	5.250"	25%	223 / 625	849	L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2271 ft-lb	6'1 5/8"	9020 ft-lb	0.252 (25%)	1.25D+1.5L	L
Unbraced	2271 ft-lb	6'1 5/8"	2271 ft-lb	1.000 (100%)	1.25D+1.5L	L
Shear	1264 lb	1 1/8"	3400 lb	0.372 (37%)	1.25D+1.5L	L
Perm Defl in.	0.042 (L/4432)	7'4 5/8"	0.516 (L/360)	0.080 (8%)	D	Uniform
LL Defl inch	0.112 (L/1664)	7'4 3/4"	0.516 (L/360)	0.220 (22%)	L	L
TL Defl inch	0.154 (L/1210)	7'4 3/4"	0.774 (L/240)	0.200 (20%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 4'10" o.c.
- 5 Bottom flange braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments	П
1	Tie-In	0-0-0 to 1-8-14	(Span)3-3-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
2	Tie-In	0-0-0 to 15-6-4	(Span)0-6-4 to 0-6-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
3	Point	1-7-6		Far Face	152 lb	404 lb	0 lb	0 lb	F10	
4	Tie-In	1-8-14 to 15-11-8	(Span)0-9-4 to 0-9-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
5	Tie-In	15-6-4 to 15-11-8	(Span)0-3-12	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
6	Point	15-8-14		Тор	32 lb	85 lb	0 lb	0 lb	J7	

Continued on page 2...

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 Uoist not to be treated with fire retardant or corre

Handling & Installation

- Handling & Installation

 1. IJoist langes must not be cut or drilled

 2. Refer to latest copy of the IJoist product information
 details for framing details, stiffener tables, web hole
 chart, bridging details, multi-ply fastening details and
 handling/erection details

 3. Damaged IJoists must not be used

 4. Design assumes top flange to be laterally restrained
 by attached sheathing or as specified in engineering
 notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length= 3.5 inches
 For flat roofs pro

This design is

READ ALL NOTES ON THIS PAGE AND ON THE

IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED

ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IN THE DESIGN OF THIS COMPONENT.

Manufacturer Info

Nascor by Kott

Kott Lumber Company 14 Anderson Blvd, Ontario Canada K2H7V1 905-642-4400

T.L. WISE

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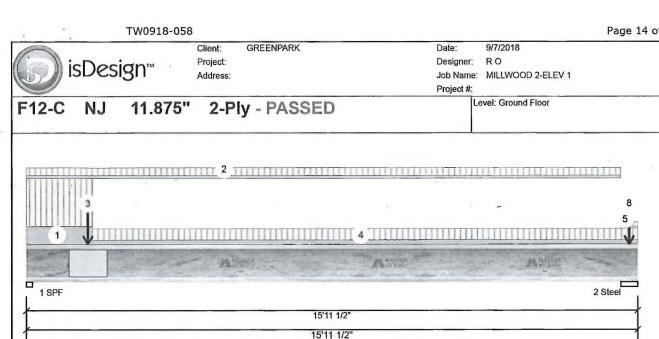
September 13, 2018





Page 2 of 2

11 7/8"



Contin	ued from page 1								
ID	Load Type	Location Trib Width	Side	Dead	Live	Snow	Wind	Comments	
7	Point	15-8-14	Тор	30 lb	80 lb	0 lb	0 lb	J7	
8	Point	15-8-14	Тор	22 lb	0 lb	0 lb	0 lb	Wall Self Weight	

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

NOtes
Calculated Shuchized Designs is responsible only of the structural adequacy of file component based on the design criteria and leadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the Intended application, and to verify the dimensions and leads.

Lumber

Dry service conditions, unless noted otherwise
 Uplish not to be treated with fire retardant or conditions.

Handling & Installation

Handling & Installation

1. Joist flanges must not be cut or drilled

2. Refer to latest copy of the I Joist product information details for framing details, stiffener tables, web hole chart. bridging details, milti-ply tastening details and handling/erection details

3. Damaged lobits must not be used

4. Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing lengther 3.5 inches
 For flat roofs provide proper drainage to prevent

This design is valid until 7/10/2021

Manufacturer Info

Nascor by Kott





Client:

Project: Address: **GREENPARK**

Date: Designer:

9/10/2018 RO

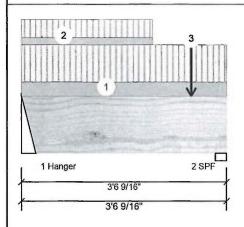
MILLWOOD 2-ELEV 1 Job Name:

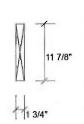
Project #

Forex 2.0E-3000Fb LVL

1,750" X 11.875" - PASSED

Level: Ground Floor





Page 1 of 1

Member Inf	ormation						Unfacto	red React	ions U	NPATTERN	ED lb	(Uplift)	
Туре:	Girder	•	Applicatio	n: Fl	oor (Resident	ial)	Brg	Live		Dead	Sno	w	Wind
Plies:	1		Design M	ethod: LS	SD		1	555		218		0	0
Moisture Cond	tion: Dry		Building C	ode: N	BCC 2010 / O	BC 2012	2	510		201		0	0
Deflection LL:	360		Load Sha	ring: No	0								
Deflection TL:	240		Deck:	Ne	ot Checked								
Importance:	Normal		Vibration:	Ne	ot Checked								
General Load													
Floor Live:	40 PSF						Bearing	s and Fact	ored	Reactions			
Dead:	15 PSF						Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
							1 - Hanger	3.000"	28%	272 / 833	1106	L	1.25D+1.5L
Analysis Res	ults						2-SPF	2.375"	40%	251 / 765	1016	L	1.25D+1.5L
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case							
Moment	785 ft-lb	1'9 5/16"	17130 ft-lb	0.046 (5%)	1.25D+1.5L	L	1					EESSIO	
Unbraced	785 ft-lb	1'9 5/16"	13259 ft-lb	0.059 (6%)	1.25D+1.5L	L					PR	3,500	AL .
Shear	745 lb	2'5 1/16"	5798 lb	0.129 (13%)	1.25D+1.5L	L				/	01		181
Perm Defl in.	0.001 (L/26669)	1'9 7/16"	0.108 (L/360)	0.010 (1%)	D	Uniform				1	86/	1 WIS	E 86
LL Defl inch	0.004 (L/10472)	1'9 3/8"	0.108 (L/360)	0.030 (3%)	L	L				(5	1	000835	66 .0
							1					100	- B

Design Notes

1 Fill all hanger nailing holes.

TL Defl inch 0.005 (L/7520)

- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.
- 4 Bottom braced at bearings.



September 13, 2018

D	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Part, Uniform	0-0-0 to 3-6-9		Тор	79 PLF	210 PLF	0 PLF	0 PLF	
2	Part. Uniform	0-0-0 to 2-3-3		Near Face	39 PLF	103 PLF	0 PLF	0 PLF	
3	Point	2-11-3		Near Face	33 lb	87 lb	0 lb	0 lb	J2
	Self Weight				5 PLF				ru Framing Squash E

Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the costractor to ensure the component suitability of the Intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or continuous.

chemicals

Handling & Installation

1. LVL beams must not be out or drilled

2. Refer to manufacturer's product information regarding installation requirements. multi-ply festeraling ideals, beam strength values, and code

3. Demonder Beams must not be used

4. Design assumes top edge is laterally restrained

5. Provide lateral support at bearing points to avoid lateral displacement and rotation

1'9 3/8" 0.161 (L/240) 0.030 (3%) D+L

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info Forex APA: PR-L318

READ ALL NOTES ON THIS PAGE AND ON THE CONTAINS SPECIFICATIONS AND CRITERIA USED This design is va

ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT IN THE DESIGN OF THIS COMPONENT.





Client:

Project:

GREENPARK

Address:

Date: 9/7/2018

RO Designer:

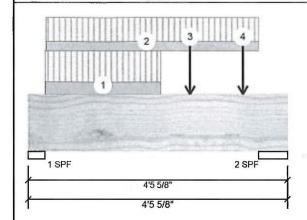
Job Name: MILLWOOD 2-ELEV 1

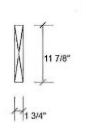
Project #:

Forex 2.0E-3000Fb LVL

1.750" X 11.875" - PASSED

Level: Ground Floor





Page 1 of 1

Member Info	rmation			Unfactor	ed React	ions U	NPATTERNI	ED lb (Uplift)	
Туре:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snow	1	Wind
Plies:	1	Design Method:	LSD	1	1024		395	C)	0
Moisture Condit	ion: Dry	Building Code:	NBCC 2010 / OBC 2012	2	1099		424	()	0
Deflection LL:	360	Load Sharing:	No							
Deflection TL:	240	Deck:	Not Checked	1						
Importance:	Normal	Vibration:	Not Checked							
General Load										
Floor Live:	40 PSF			Bearings	and Fac	tored l	Reactions			
Dead:	15 PSF			Bearing I	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
				1-SPF	3.500"	54%	493 / 1536	2029	L	1.25D+1.5L
				2-SPF :	5.875"	34%	529 / 1649	2178	L	1.25D+1.5L

Analysis Results

	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
	Moment	2002 ft-lb	2'1 9/16"	17130 ft-lb	0.117 (12%)	1.25D+1.5L	L
	Unbraced	2002 ft-lb	2'1 9/16"	11720 ft-lb	0.171 (17%)	1.25D+1.5L	L
	Shear	2314 lb	3' 5/8"	5798 lb	0.399 (40%)	1.25D+1.5L	L
	Perm Defl in.	0.004 (L/10769)	2'1 11/16"	0.127 (L/360)	0.030 (3%)	D	Uniform
	LL Defl inch	0.011 (L/4138)	2'1 11/16"	0.127 (L/360)	0.090 (9%)	L	L
	TL Defl inch	0.015 (L/2989)	2'1 11/16"	0.191 (L/240)	0.080 (8%)	D+L	L
ī							



Design Notes

- 1 Girders are designed to be supported on the bottom edge only
- 3 Bottom braced at bearings.

,	chacle are addigited to be deposited on the bottom edge only.
2	Top braced at hearings

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Part. Uniform	0-3-8 to 2-3-8		Near Face	120 PLF	319 PLF	0 PLF	0 PLF	
2	Part. Uniform	0-3-12 to 3-11-12		Тор	90 PLF	240 PLF	0 PLF	0 PLF	
3	Point	2-9-8		Near Face	115 lb	305 lb	0 lb	0 lb	J7
4	Point	3-8-8		Near Face	112 lb	300 lb	0 lb	0 lb	J7
	Self Weight				5 PLF			Pass-Th	ru Framing

Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the Intended application, and to verify the dimensions and loads. Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corre

Handling & Installation

- Handling & Installation

 1. UVL beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

This design is

Manufacturer Info Forex APA: PR-L318

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT **CONTAINS SPECIFICATIONS AND CRITERIA USED** IN THE DESIGN OF THIS COMPONENT.



Client:

Project: Address: GREENPARK

9/7/2018 Date: Designer:

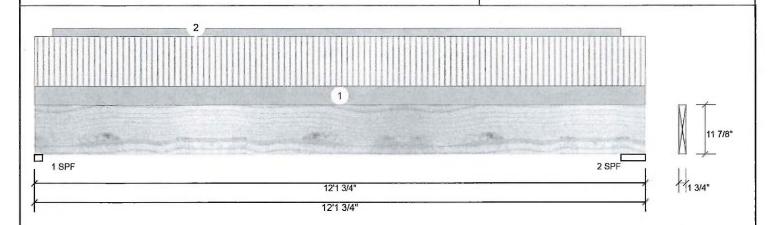
Job Name: MILLWOOD 2-ELEV 1

Project #:

Forex 2.0E-3000Fb LVL F5-A

1,750" X 11,875" - PASSED

Level: Ground Floor



Member Inforn	nation			Unfactor	red Reac	tions U	NPATTERNI	ED lb (Uplift)	
Type:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snov	V	Wind
Plies:	1	Design Method:	LSD	1	73		67	()	0
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012	2	77		70	()	0
Deflection LL:	360	Load Sharing:	No							
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal	Vibration:	Not Checked							
General Load										
Floor Live:	40 PSF			Bearings	and Fac	tored F	Reactions			
Dead:	15 PSF			Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.
				1-SPF	1.875"	10%	83 / 109	192	L	1.25D+1.5L
				2-SPF	5.875"	3%	88 / 115	203	L	1.25D+1.5L

Analysis Results

Actual	Location	Allowed	Capacity	Comb.	Case
553 ft-lb	5'10 7/8"	17130 ft-lb	0.032 (3%)	1.25D+1.5L	L
553 ft-lb	5'10 7/8"	3868 ft-lb	0.143 (14%)	1.25D+1.5L	L
158 lb	1'1"	5798 lb	0.027 (3%)	1.25D+1.5L	L
0.011 (L/13107)	5'10 7/8"	0.388 (L/360)	0.030 (3%)	D	Uniform
0.012 (L/12101)	5'10 7/8"	0.388 (L/360)	0.030 (3%)	L	L
0.022 (L/6292)	5'10 7/8"	0.581 (L/240)	0.040 (4%)	D+L	L
	553 ft-lb 553 ft-lb 158 lb 0.011 (L/13107) 0.012 (L/12101)	553 ft-lb 5'10 7/8" 553 ft-lb 5'10 7/8" 158 lb 1'1" 0.011 5'10 7/8" (L/13107) 0.012 5'10 7/8"	553 ft-lb 5'10 7/8" 17130 ft-lb 553 ft-lb 5'10 7/8" 3868 ft-lb 158 lb 1'1" 5798 lb 0.011 5'10 7/8" 0.388 (L/360) (L/13107) 0.012 5'10 7/8" 0.388 (L/360) (L/12101)	553 ft-lb 5'10 7/8" 17130 ft-lb 0.032 (3%) 553 ft-lb 5'10 7/8" 3868 ft-lb 0.143 (14%) 158 lb 1'1" 5798 lb 0.027 (3%) 0.011 (L/13107) 5'10 7/8" 0.388 (L/360) 0.030 (3%) 0.012 (L/12101) 5'10 7/8" 0.388 (L/360) 0.030 (3%)	553 ft-lb 5'10 7/8" 17130 ft-lb 0.032 (3%) 1.25D+1.5L 553 ft-lb 5'10 7/8" 3868 ft-lb 0.143 (14%) 1.25D+1.5L 158 lb 1'1" 5798 lb 0.027 (3%) 1.25D+1.5L 0.011 (L/13107) 5'10 7/8" 0.388 (L/360) 0.030 (3%) D 0.012 (L/12101) 5'10 7/8" 0.388 (L/360) 0.030 (3%) L

T.L. WISE 100083566 TWCE OF ONTE September 13, 2018

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Top braced at bearings.
- 3 Bottom braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 12-1-12	(Span)0-7-6	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Part. Uniform	0-4-6 to 11-7-15		Тор	2 PLF	0 PLF	0 PLF	0 PLF	
	Self Weight				5 PLF				

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the inlended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- and ling & Installation
 LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, mutil-pit
 fastening details, beam strength values, and code
 approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

For flat roofs provide proper drainage to pre-ponding

Manufacturer Info Forex APA: PR-L318

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT. This design is

Kott Lumber Company 14 Anderson Blvd, Ontario Canada K2H7V1 905-642-4400

Version 18.40.162 Powered by iStruct™



Client: **GREENPARK**

Project: Address:

Date: 9/7/2018

RO Designer:

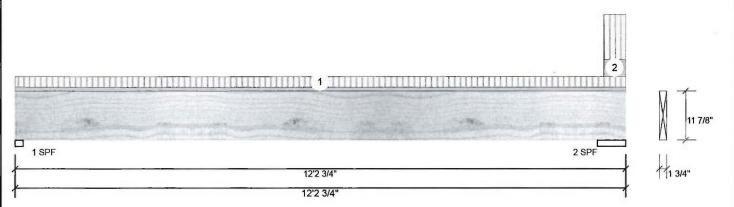
Job Name: MILLWOOD 2-ELEV 1

Project #

Forex 2.0E-3000Fb LVL

1.750" X 11.875" - PASSED

Level: Ground Floor



Nember Infor	mation			Unfactor	ed Reac	tions U	NPATTERN	ED lb (Uplift)	
Туре:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snov	N	Wind
Plies:	1	Design Method:	LSD	1	67		53		0	0
Moisture Conditio	n: Dry	Building Code:	NBCC 2010 / OBC 2012	2	95		66		0	0
Deflection LL:	360	Load Sharing:	No							
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal	Vibration:	Not Checked							
General Load				,						
Floor Live:	40 PSF			Bearings	and Fac	tored l	Reactions			
Dead:	15 PSF			Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
				1 - SPF	1.875"	8%	67 / 101	168	L	1.25D+1.5L
		L		2-SPF	6.875"	3%	82 / 142	224	L	1.25D+1.5L

Analysis Results

	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
ı	Moment	480 ft-lb	5'10 7/8"	17130 ft-lb	0.028 (3%)	1.25D+1.5L	L
	Unbraced	480 ft-lb	5'10 7/8"	3868 ft-lb	0.124 (12%)	1.25D+1.5L	L
	Shear	137 lb	1'1"	5798 lb	0.024 (2%)	1.25D+1.5L	L
	Perm Defl in.	0.008 (L/16496)	5'10 7/8"	0.388 (L/360)	0.020 (2%)	D	Uniform
	LL Defl inch	0.011 (Ľ/13065)	5'10 7/8"	0.388 (L/360)	0.030 (3%)	L	L
	TL Defl inch	0.019 (L/7290)	5'10 7/8"	0.581 (L/240)	0.030 (3%)	D+L	L
_							



Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Top braced at bearings.
- 3 Bottom braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 12-2-12	(Span)0-6-14	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	11-9-6 to 12-2-12	(Span)2-6-2	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	Self Weight				5 PLF				

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or con

- Handling & Installation

 1. LVL beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Demaged Beams must not be used

 1. Detailer servings for soften is laterally cardinated.
- Damaged Beams must not be used Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation

This design is

Manufacturer Info

Forex APA: PR-L318

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Client: **GREENPARK**

Project: Address: Date:

Designer: RO

Job Name: MILLWOOD 2-ELEV 1

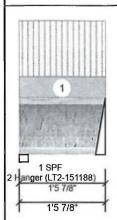
9/7/2018

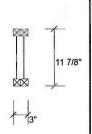
Project #

11.875" NJ

2-Ply - PASSED

Level: Ground Floor





Page 1 of 1

Member Inforr	nation						Unfacto	red React	tions U	NPATTERNI	ED lb (Uplift)	
Type:	Girder		Application:	F	loor (Resident	ial)	Brg	Live		Dead	Sno	N	Wind
Plies:	2		Design Meth	od: L	SD		1	48		18		0	0
Moisture Condition	: Dry		Building Cod	e: N	BCC 2010 / O	BC 2012	2	49		18		0	0
Deflection LL:	360		Load Sharing	g: N	o								
Deflection TL:	240		Deck:	N	ot Checked		1						
Importance:	Normal		Vibration:	N	ot Checked								
General Load													
Floor Live:	40 PSF						Bearing	s and Fac	tored	Reactions			
Dead:	15 PSF						Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
							1-SPF	1.875"	4%	23/72	95	L	1.25D+1.5L
Analysis Result	-						2 - Hanger	2.000"	4%	23 / 73	96	L	1.25D+1.5L
		1	Allermad		Ozusk	0	nanger						
Analysis Ac		Location		apacity	Comb.	Case					-		
	ft-lb	8 7/8"		003 (0%)	1.25D+1.5L		1				10	FESSIO,	Va.
Unbraced 27	ft-lh	8 7/R"	8539 ft-lb 0 (10%1 200	1 25D+1 5I	1					100	A STATE OF THE PARTY.	100

l	Moment	27 ft-lb	8 7/8"	9020 ft-lb	0.003 (0%)	1.25D+1.5L	I
l	Unbraced	27 ft-lb	8 7/8"	8539 ft-lb	0.003 (0%)	1.25D+1.5L	I
l	Shear	83 lb	1 1/8"	3400 lb	0.024 (2%)	1.25D+1.5L	I
l	Perm Defl in.	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
l	LL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
l	TL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
۲							

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top flange braced at bearings.
- 6 Bottom flange braced at bearings.

10	PROFESSIONAL
LICENS	T.L. WISE 100083566
13	SUD STO

September 13, 2018

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-5-14	(Span)3-3-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. If is the responsibility of the customer and/or the contractor to ensure the component suitability of the Inlended application, and to verify the dimensions and loads. Lumber

Dry service conditions, unless noted otherwise Uoist not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. Just finages must not be cut or drilled

 2. Refer to latest copy of the IJoist product information
 details for framing details, stiffener tables, web note
 chart, bridging details, multi-ply fastening details and
 handling/erection details

 3. Damaged fubrist must not be used

 4. Design assumes top flange to be laterally restrained
 by attached sheathing or as specified in engineering
 notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing antibers 3.5 Inches.
 For flat roofs pro

ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

This design is

READ ALL NOTES ON THIS PAGE AND ON THE

Manufacturer Info

Nascor by Kott





GREENPARK Client:

Project: Address

9/7/2018 Date:

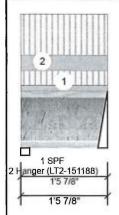
RO Designer:

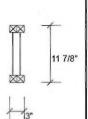
Job Name: MILLWOOD 2-ELEV 1

Project #:

11.875" 2-Ply - PASSED NJ

Level: Ground Floor





Page 1 of 1

Member Inform	nation				Unfacto	red React	ions U	INPATTERNI	ED lb (Uplift)	
Type:	Girder		Application:	Floor (Residential)	Brg	Live		Dead	Snow	Wind
Plies:	2		Design Method:	LSD	1	65		24	0	0
Moisture Condition	Dry		Building Code:	NBCC 2010 / OBC 2012	2	66		25	0	0
Deflection LL:	360		Load Sharing:	No						
Deflection TL:	240		Deck:	Not Checked						
Importance:	Normal		Vibration:	Not Checked						
General Load										
Floor Live:	40 PSF				Bearing	s and Fac	tored l	Reactions		
Dead:	15 PSF				Bearing	Length	Cap.	React D/L lb	Total Ld. Case	Ld. Comb.
					1 - SPF	1.875"	5%	31 / 98	128 L	1.25D+1.5L
Analysis Result	s				2 - Hanger	2.000"	5%	31/99	130 L	1.25D+1.5L
Analysis Act	ual	Location	Allowed Capac	ity Comb. Case						-
Moment 361	t-lb	8 7/8"	9020 ft-lb 0.004 (0%) 1.25D+1.5L L					OFESSIO	N. T.

-	9020 ft-lb 8539 ft-lb	0.004 (0%)	1,25D+1.5L	
8"	8539 ft-lb	0.004 (0%)	4 25D . 4 5I	
			1.23D+1.5L	L
8"	3400 lb	0.033 (3%)	1.25D+1.5L	L
0	999.000 (L/0)	0.000 (0%)		
0	999.000 (L/0)	0.000 (0%)		
0	999.000 (L/0)	0.000 (0%)		
	0	0 999.000 (L/0)	0 999.000 (L/0) 0.000 (0%) 0 999.000 (L/0) 0.000 (0%) 0 999.000 (L/0) 0.000 (0%)	0 999.000 (L/0) 0.000 (0%)



September 13, 2018

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top flange braced at bearings.
- 6 Bottom flange braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-5-14	(Span)1-1-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 1-5-14	(Span)3-3-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the coeractor to ensure the component suitability of the Intended application, and to verify the dimensions and loads. Lumber

Dry service conditions, unless noted otherwise
 Unist not to be treated with fire relardant or corrosive

Handling & Installation

- Handling & Installation

 1. Juist flanges must not be cut or drilled

 2. Refer to latest copy of the Juist product information details for framing details, stiffener tables, web hote chart. bridging details, multi-ply fastening details and handling/eraction details

 3. Damaged Juists must not be used

 4. Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

 Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing le-util>= 3.5 Inches
 For fat roofs provided to the ponding Provided Tools of Provided Tools provi Nascor by Kott

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Manufacturer Info

This design is



isDesign™

Client:

GREENPARK

Project: Address:

9/10/2018 Date:

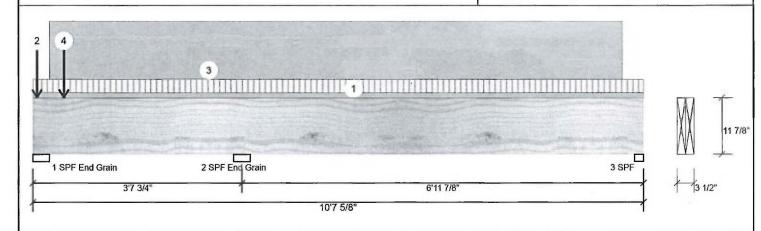
Designer: RO

Job Name: MILLWOOD 2-ELEV 1

Project #:

1.750" X 11.875" F15-A Forex 2.0E-3000Fb LVL

2-Ply - PASSED Level: Ground Floor



Type:	Girder	Application:	Floor (Residential)
Plies:	2	Design Method:	LSD
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		

Analysis Results										
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case				
Neg Moment	-617 ft-lb	3'7 3/4"	22269 ft-lb	0.028 (3%)	1.4D	Uniform				
Unbraced	-617 ft-lb	3'7 3/4"	22269 ft-lb	0.028 (3%)	1.4D	Uniform				
Pos Moment	521 ft-lb	7'9"	22269 ft-lb	0.023 (2%)	1.4D	Uniform				
Unbraced	521 ft-lb	7'9"	21873 ft-lb	0.024 (2%)	1.4D	Uniform				
Shear	421 lb	4'7 5/8"	7537 lb	0.056 (6%)	1.4D	Uniform				
Perm Defl in.	0.005 (L/17692)	7'3 7/16"	0.230 (L/360)	0.020 (2%)	D	Uniform				
LL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)						
TL Defl inch	0.006 (L/14724)	7'3 3/8"	0.345 (L/240)	0.020 (2%)	D+L	JL				

Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral stenderness ratio based on full section width.

Unfactored Reactions UNPATTERNED Ib (Uplift)

Live	Dead	Snow	Wind
986	484	0	0
176	679	0	0
52	259	0	0
	986 176	986 484 176 679	986 484 0 176 679 0

Bearings and Factored Reactions

Bearing Len	gth Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF 3.50 End Grain	0" 24%	578 / 1504	2082	L_	1.25D+1.5L
2 - SPF 3.50 End Grain	0" 17%	995/0	995	Uniform	1.4D
3 - SPF 1.87	5" 13%	348 / 0	348	Uniform	1.4D



September 13, 2018

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Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or con

chemicals

Handling & Installation

andling & Installation
LVL beams must not be cut or drilled
Refer to manufacturer's product information
regarding installation requirements, multi-ply
fastening details, beam strength values, and code
approvals
Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

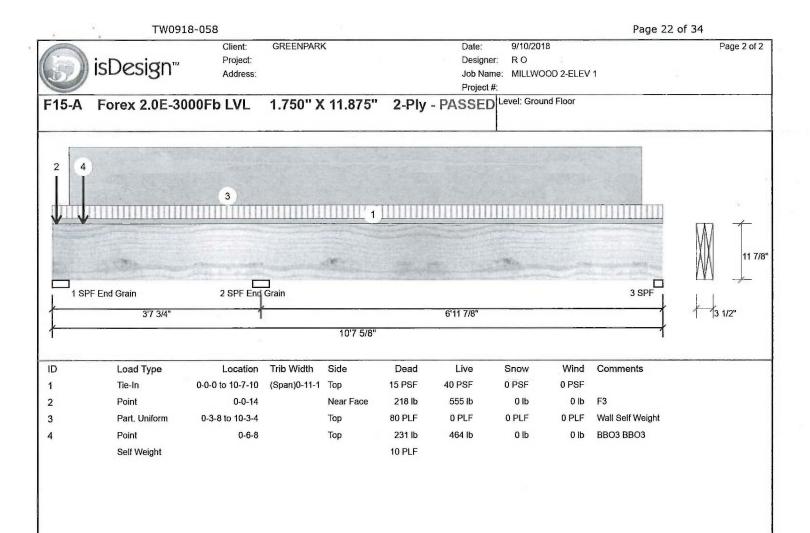
6. For flat roofs provide proper drainage to prevent

This design is

Manufacturer Info Forex APA: PR-L318

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Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

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Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

and Iling & Installation.

LVL beams must not be cut or drilled.
Refer to manufacturer's product Information regarding installation requirements, mutil-ply fastening details, beam strength values, and code approvals.

Damaged Beams must not be used.
Design assumes top edge is laterally restrained.
Provide lateral support at bearing points to avoid lateral displacement and rotation.

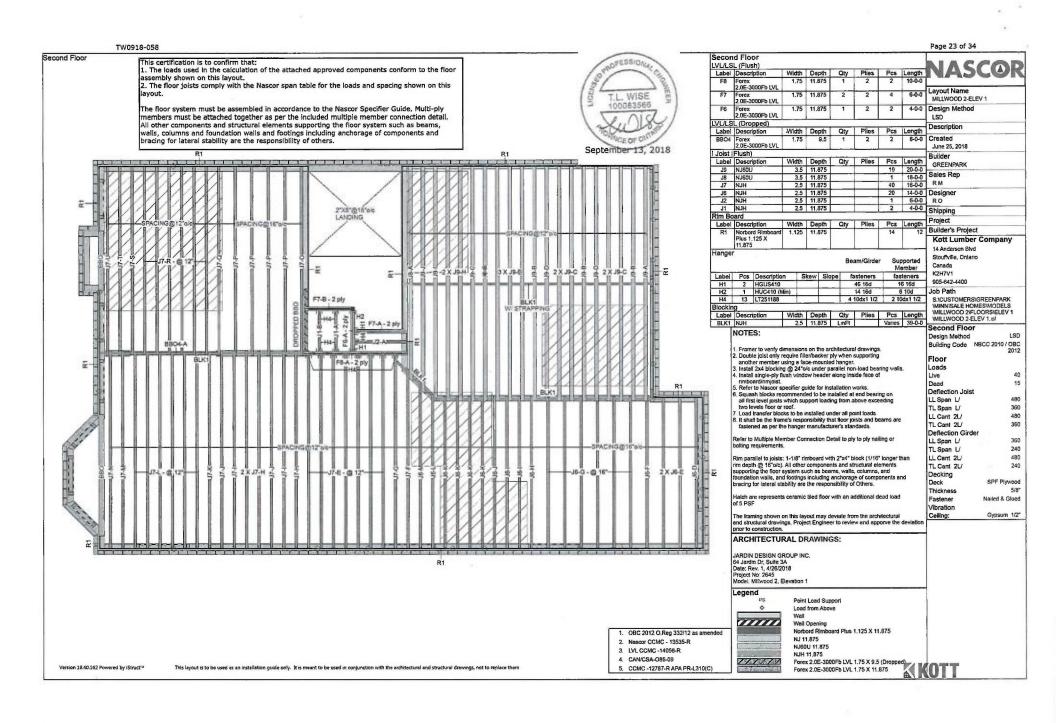
For flat roofs provide proper drainage to prevent ponding

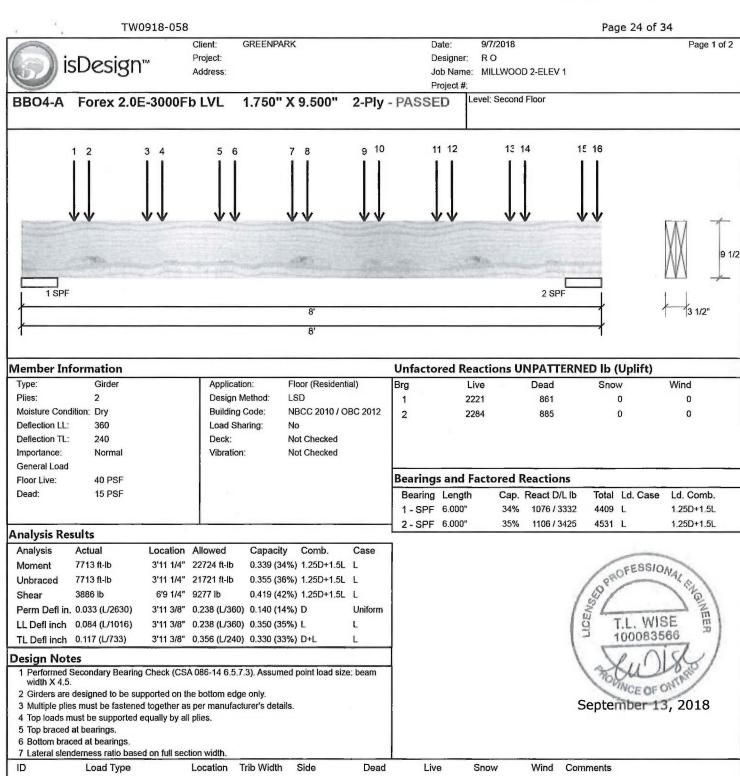
This design is valid until 7/10/2021

Forex

APA: PR-L318

Manufacturer Info





Lateral	Sicilacificas fallo basca on	Juli Scottori Width.							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Point	0-8-12		Тор	111 lb	297 lb	0 lb	0 lb	J7
2	Point	0-11-4		Тор	116 lb	310 lb	0 lb	0 lb	J7
3	Point	1-8-12		Тор	111 lb	297 lb	0 lb	0 lb	J7
4	Point	1-11-4		Тор	116 lb	310 lb	0 lb	0 lb	J7
5	Point	2-8-12		Тор	111 lb	297 lb	0 lb	0 lb	J7

Continued on page 2...

Notes

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Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or con

Handling & Installation

L.VI. beams must not be cut or drilled
 Refer to manufacturer's product information regarding installation requirements, multi-ply fastenting details, beam strength values, and code approvals
 Damaged Beams must not be used

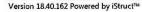
Design assumes top edge is laterally restrelned Provide lateral support at bearing points to avoid lateral displacement and rotation

This design is

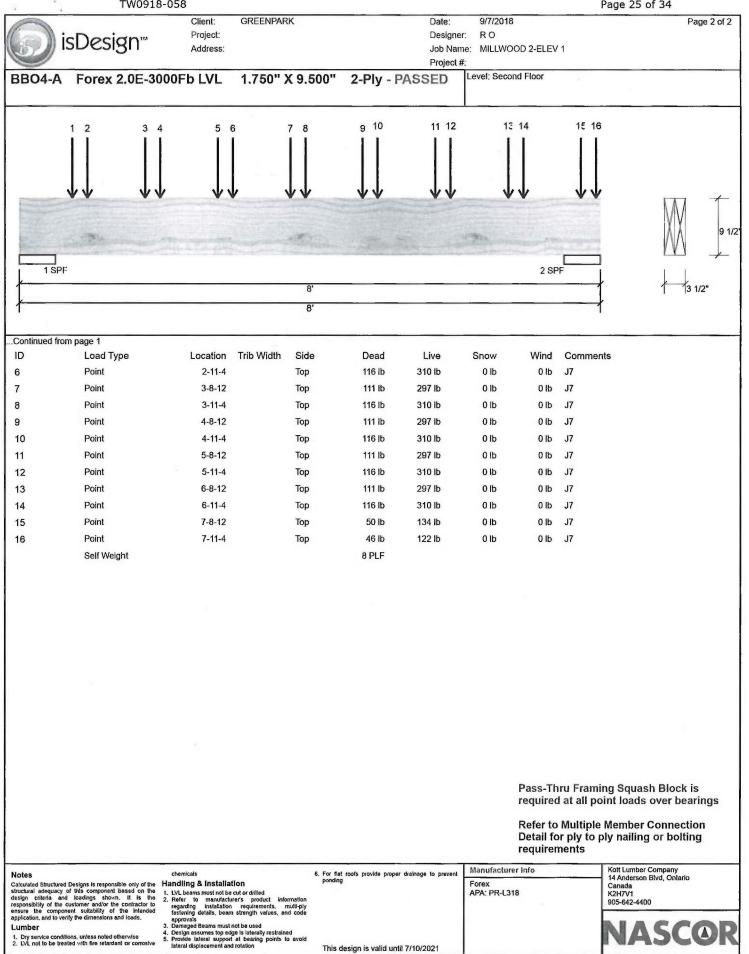
Manufacturer Info Forey APA: PR-L318

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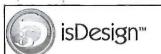








This design is valid until 7/10/2021



Client:

Project:

GREENPARK

9/7/2018 Date: Designer:

Job Name: MILLWOOD 2-ELEV 1

Project #:

Forex 2.0E-3000Fb LVL F6-A

1.750" X 11.875"

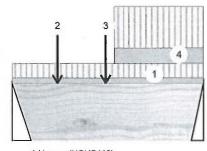
2-Ply - PASSED

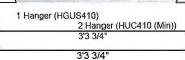
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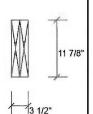
1

2

Level: Second Floor







Wind

0

0

0

0

Page 1 of 1

Member Info	rmation		
Type:	Girder	Application:	Floor (Residential)
Plies:	2	Design Method:	LSD
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		
	1-1-		

Bearings and Factored Reactions										
Bearing Length	Cap. React D/L lb	Total	Ld. Case	Ld. Comb.						

Dead

83

88

Unfactored Reactions UNPATTERNED Ib (Uplift)

150

169

Analysis Results Analysis Actual Location Allowed Capacity Comb. Case 246 ft-lb 1'7 1/2" 34261 ft-lb 0.007 (1%) 1.25D+1.5L L Moment 0.007 (1%) 1.25D+1.5L L 1'7 1/2" 34261 ft-lb 246 ft-lb Unbraced 0.021 (2%) 1.25D+1.5L L 246 lb 1'3 1/8" 11596 lb Shear Perm Defl in. 0.000 (L/999) 0 999.000 (L/O) 0.000 (0%) LL Defl inch 0.000 (L/999) 0 999.000 (L/0) 0.000 (0%) TL Defl inch 0.001 1'7 3/4" 0.145 (L/240) 0.010 (1%) D+L L (L/46486)

1.25D+1.5L 4.000" 3% 104 / 225 329 L Hanger 1.25D+1.5L 2 500" 6% 110 / 253 363 1 2 -Hanger

> T.L. WISE 100083566 100083566 TACE OF ONTP

September 13, 2018

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on full section width.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	
1	Tie-In	0-0-0 to 3-3-12	(Span)1-4-8	Тор	15 PSF	40 PSF	0 PSF	
2	Point	0-9-7		Near Face	26 lb	70 lb	0 lb	
3	Point	1-7-8		Near Face	33 lb	35 lb	0 lb	
4	Tie-In	1-9-4 to 3-3-12	(Span) 3-11-13	Тор	15 PSF	40 PSF	0 PSF	
	Self Weight				10 PLF			

wo	Wind	Comments
PSF	0 PSF	
0 lb	0 lb	J2
0 lb PSF	PassaTh required	ru _z Framing Squash Block is I at all point loads over bearings
OI .	Refer to	Multiple Member Connection or ply to ply nailing or bolting nents

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the Intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrost

Handling & Installation

- and ling & Installation.

 LVL beams must not be cut or drilled.
 Refer to manufacturer's product Information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals.

 Demaged Beams must not be used.
 Design assumes to edge is laterally restrained.
 Provide lateral support at bearing points to avoid lateral displacement and rotation.

6. For flat roofs provide proper drainage to prevent

This design is

Manufacturer Info Forex APA: PR-L318

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



isDesign™

Client:

Address:

GREENPARK Project:

Date: 9/7/2018

Designer: RO

Job Name: MILLWOOD 2-ELEV 1

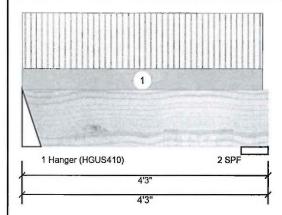
Project #:

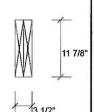
Forex 2.0E-3000Fb LVL

1.750" X 11.875"

2-Ply - PASSED

Level: Second Floor





Page 1 of 1

Viember Inf	ember Information						Unfactored Reactions UNPATTERNED Ib (Uplift)						
Туре:	Girder		Application	n: F	oor (Resident	ial)	Brg	Live		Dead	Sno	w	Wind
Plies:	2		Design M	ethod: L	SD		1	35		33		0	0
Moisture Condi	ition: Dry		Building (Code: N	BCC 2010 / O	BC 2012	2	35		34		0	0
Deflection LL:	360		Load Sha	ring: N	0		-						
Deflection TL:	240		Deck:	N	ot Checked								
Importance:	Normal		Vibration:	N	ot Checked								
General Load													
Floor Live:	40 PSF						Bearing	s and Fact	ored l	Reactions			
Dead:	15 PSF						Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
							1 - Hanger	4.000"	1%	41 / 52	93	L	1.25D+1.5L
Analysis Res	ults						2-SPF	5.500"	1%	43 / 53	95	L	1.25D+1.5L
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case							-
Moment	72 ft-lb	2' 3/4"	34261 ft-lb	0.002 (0%)	1.25D+1.5L	L	1				/	OFESSIO	Ma
Unbraced	72 ft-lb	2' 3/4"	34261 ft-lb	0.002 (0%)	1.25D+1.5L	L					108		10
Shear	36 lb	1'3 1/8"	11596 lb	0.003 (0%)	1.25D+1.5L	L				/	18 L		161
Perm Defl in.	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)						1	3 1		SE S
LL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)						- 1	LICE	T.L. WK	SE m
TI Definal	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)						1	.al	1000835	66

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on full section width.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 4-1-14	(Span)0-10-1	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	Self Weight				10 PLF				

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. If is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

andling & Installation

LVL beams must not be cut or drilled

Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, end code approvals

Damaged Beams must not be used

Design assumes top edge is laterally restrained

Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

This design is

Manufacturer Info Forex APA: PR-L318

IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IN THE DESIGN OF THIS COMPONENT.

Kott Lumber Company 14 Anderson Blvd, Ontario Canada K2H7V1 905-642-4400

September 13, 2018



isDesign[™]

Client:

Project: Address: **GREENPARK**

Date: 9/7/2018

Designer: RO

Job Name: MILLWOOD 2-ELEV 1

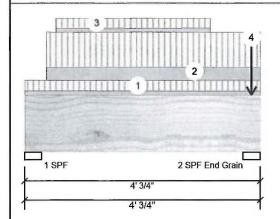
Project #:

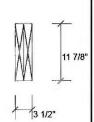
Forex 2.0E-3000Fb LVL

1.750" X 11.875"

2-Ply - PASSED

Level: Second Floor





Page 1 of 1

Member Info	rmation		
Туре:	Girder	Application:	Floor (Residential)
Plies:	2	Design Method:	LSD
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		

Brg	Live	Dead	Snow	Wind
1	643	260	0	0
2	885	375	0	0

Analysis Results Analysis Actual Location Allowed Capacity Comb. Case Moment 1215 ft-lb 2' 5/16" 34261 ft-lb 0.035 (4%) 1.25D+1.5L L Unbraced 1215 ft-lb 2' 5/16" 34261 ft-lb 0.035 (4%) 1.25D+1.5L L Shear 711 lb 2'10 1/8" 11596 lb 0.061 (6%) 1.25D+1.5L L Perm Defl in. 0.001 2' 5/16" 0.120 (L/360) 0.010 (1%) D Uniform (L/34561) 0.003 2' 5/16" 0.120 (L/360) 0.030 (3%) L LL Defl inch (L/13799) TL Defl inch 0.004 (L/9862) 2' 5/16" 0.180 (L/240) 0.020 (2%) D+L

Bearings and Factored Reactions

Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.	
1 - SPF	3.500"	17%	325 / 965	1289	L	1.25D+1.5L	
2 - SPF End Grain	3,500"	20%	469 / 1328	1797	L	1.25D+1.5L	



- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on full section width.



ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 4-0-12	(Span)3-7-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Part. Uniform	0-4-8 to 4-0-12		Тор	90 PLF	240 PLF	0 PLF	0 PLF	
3	Part. Uniform	0-6-8 to 3-2-8		Near Face	25 PLF	68 PLF	0 PLF	PasseTh	nru Framing Squash Block is
4	Point	3-11-0		Near Face	88 lb	169 lb	0 lb	require	d at all point loads over bearings
	Self Weight				10 PLF				Multiple Member Connection or ply to ply nailing or bolting

This design is

nnection bolting requirements

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the Intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or co

Handling & Installation

- LVL beams must not be cut or drilled
 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- approvals Damaged Beams must not be used
- Damaged Beams must not be used Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation

Manufacturer Info Forex APA: PR-L318

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Client:

Project: Address: GREENPARK

9/7/2018 Date: Designer:

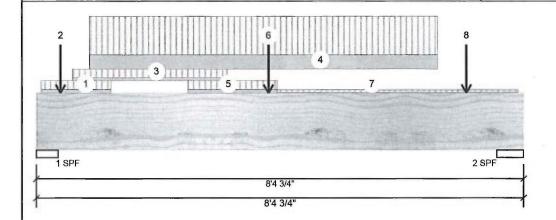
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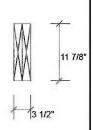
MILLWOOD 2-ELEV 1 Job Name:

Project #:

Forex 2.0E-3000Fb LVL F8-A

1.750" X 11.875" 2-Ply - PASSED Level: Second Floor





Wind

Page 1 of 2

Mellipei Illioilliation							
Туре:	Girder						
Plies:	2						
Moisture Condition:	Dry						
Deflection LL:	360						
Deflection TL:	240						
Importance:	Normal						
General Load							

40 PSF

15 PSF

Member Information

Application: Floor (Residential) Design Method: LSD **Building Code:**

Vibration:

NBCC 2010 / OBC 2012

Not Checked

Brg

Load Sharing: No Deck: Not Checked

Unfactored Reactions UNPATTERNED	lb	(Uplift)
---	----	----------

1	1594	649	0	0
'	1377	568		0
4	13//	500	0	U

Dead

Snow

Bearings and Factored Reactions

I ive

Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.	
1 - SPF	4.500"	33%	811 / 2391	3202	L	1.25D+1.5L	
2-SPF	5.500"	23%	710 / 2066	2776	L	1.25D+1.5L	

Analysis Results

Floor Live:

Dead:

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	5966 ft-lb	4'	34261 ft-lb	0.174 (17%)	1.25D+1.5L	L
Unbraced	5966 ft-lb	4'	31511 ft-lb	0.189 (19%)	1.25D+1.5L	L
Shear	3035 lb	7' 1/8"	11596 lb	0.262 (26%)	1.25D+1.5L	L
Perm Defl in.	0.016 (L/5638)	4' 3/4"	0.256 (L/360)	0.060 (6%)	D	Uniform
LL Defl inch	0.040 (L/2325)	4' 13/16"	0.256 (L/360)	0.150 (15%)	L	L
TL Defl inch	0.056 (L/1646)	4' 13/16"	0.384 (L/240)	0.150 (15%)	D+L	L

Design Notes



September 13, 2018

1 Girders are designed to be supported on the bottom edge only.

- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on full section width.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-1-0 to 1-3-8	(Span)3-7-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	0-5-0		Near Face	77 lb	205 lb	0 lb	0 lb	J7
3	Part. Uniform	0-7-8 to 3-3-8		Far Face	25 PLF	68 PLF	0 PLF	0 PLF	
4	Part. Uniform	0-11-0 to 6-11-0		Near Face	115 PLF	308 PLF	0 PLF	0 PLF	
5	Tie-In	2-7-8 to 4-1-12	(Span)3-7-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
6	Point	4-0-0		Far Face	83 lb	150 lb	0 lb	0 lb	F6

Continued on page 2...

Notes

Celculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the Intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or con

Handling & Installation

LVL beams must not be cut or drilled
 Refer to menufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code

approvals
Damaged Beams must not be used
Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

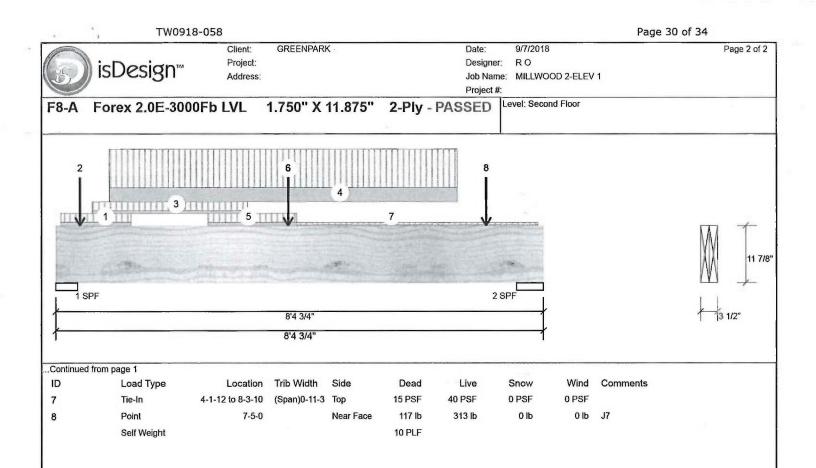
Manufacturer Info Forex APA: PR-L318

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Kott Lumber Company 14 Anderson Blvd, Ontario Canada K2H7V1 905-642-4400



This design is



Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and leadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the Intended application, and to verify the dimensions and loads. Lumber

Handling & Installation

LVL beams must not be cut or drilled
Refer to manufacturer's product information
regarding installation requirements, multi-ply
fastening details, beam strength values, and code

approvals
Damaged Beams must not be used
Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

This design is valid until 7/10/2021

Forex APA: PR-L318

Manufacturer Info



Client:

GREENPARK

Project: Address:

Date: 9/7/2018

Designer: RO

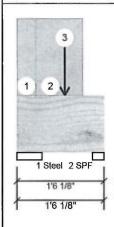
Job Name: MILLWOOD 2-ELEV 2

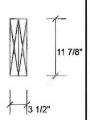
Project #

Forex 2.0E-3000Fb LVL F15-A

1.750" X 11.875"

2-Ply - PASSED Level: Second Floor





Page 1 of 1

Member Info	ember Information				Unfactored Reactions UNPATTERNED Ib (Uplift)					
Туре:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow		Wind	
Plies:	2	Design Method:	LSD	1	72	103	0		0	
Moisture Conditi	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	61	53	0		0	
Deflection LL:	360	Load Sharing:	No							
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal	Vibration:	Not Checked	1						
General Load										
Floor Live:	40 PSF			Bearings ar	nd Factored	Reactions				
Dead:	15 PSF			Bearing Le	ngth Cap	React D/L lb	Total L	d. Case	Ld. Comb.	
				1 - Steel 5.2	50" 2%	129 / 108	237 L		1.25D+1.5L	
				2-SPF 2.3	75" 3%	66 / 92	158 L		1.25D+1.5L	

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case	7
Moment	78 ft-lb	10"	33233 ft-lb	0.002 (0%)	1.25D+1.5L	L	
Unbraced	78 ft-lb	10"	33233 ft-lb	0.002 (0%)	1.25D+1.5L	L	
Shear	107 lb	1'4 3/8"	11248 lb	0.009 (1%)	0.9D+1.5L	L	
Perm Defl in.	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)			
LL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)			
TL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)			

D

- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral clanderness ratio based on full section width

Shear	107 lb	1'4 3/8"	11248 lb	0.009 (1%)	0.9D+1.5L	L	
Perm Defl in	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)			
LL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)			
TL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)			
Design Not	es						
1 Girders are	designed to be sup	ported on the	ne bottom edge	only.			
2 Multiple plie	es must be fastened	d together as	s per manufacti	ırer's details.			

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September 13, 2018

U Laterar	sicilacilicas latio pascu t	il iuli section wiutii.							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Part. Uniform	0-0-0 to 0-4-0		Тор	80 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
2	Part. Uniform	0-4-0 to 1-1-12		Тор	80 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
3	Point	0-10-0		Near Face	50 lb	133 lb	0 lb	0 lb	J10
	Self Weight				10 PLF				nru Framing Squash Block is d at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the Inlended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled
 Refer to manufacturer's product information regarding instalfablion requirements, multi-ply fastening details, beam strength values, and code
- approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info Forex APA: PR-L318

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED This design IN THE DESIGN OF THIS COMPONENT.



Client:

Project: Address: **GREENPARK**

Date: 9/7/2018

Designer: RO

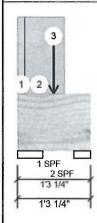
Job Name: MILLWOOD 2-ELEV 3

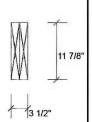
Project #:

F15-A Forex 2.0E-3000Fb LVL

1.750" X 11.875"

2-Ply - PASSED Level: Second Floor





Page 1 of 1

Member Information					Unfactored Reactions UNPATTERNED Ib (Uplift)						
Type:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Sno	w	Wind	
Plies:	2	Design Method:	LSD	1	108		101		0	0	
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	65		41		0	0	
Deflection LL:	360	Load Sharing:	No								
Deflection TL:	240	Deck:	Not Checked	1							
Importance:	Normal	Vibration:	Not Checked								
General Load											
Floor Live:	40 PSF			Bearing:	s and Fac	tored	Reactions				
Dead:	15 PSF			Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.	
				1 - SPF	5.250"	3%	126 / 162	288	L	1.25D+1.5L	
				2-SPF	3.500"	2%	52 / 98	149	L	1.25D+1.5L	

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case	
Moment	58 ft-lb	7 1/2"	34261 ft-lb	0.002 (0%)	1.25D+1.5L	L	
Unbraced	58 ft-lb	7 1/2"	34261 ft-lb	0.002 (0%)	1.25D+1.5L	L	
Shear	190 lb	1'4 3/8"	11596 lb	0.016 (2%)	1.25D+1.5L	L	
Perm Defl in.	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)			
LL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)			
TL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)			
							_

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on full section width

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September 13, 2018

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ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Part. Uniform	0-0-0 to 0-1-8		Тор	80 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
2	Part. Uniform	0-1-8 to 0-9-12		Тор	80 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
3	Point	0-7-8		Far Face	65 lb	173 lb	0 lb	0 lb	J9
	Self Weight				10 PLF				nru Framing Squash Block is d at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shoom. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the Inlended application, and to verify the dimensions and loads.

Lumber

Handling & Installation

- Handling & Installation

 1. UV. beams must not be cut or drilled

 2. Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
 approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid
 lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info Forex APA: PR-L318

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Kott Lumber Company 14 Anderson Blvd, Ontario Canada K2H7V1

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