

NE0818-059 GREENPARK-MINNISALE HOMES-GRANDBROOKE 12-ELEV 1 & 2 Second Floor Second Floor REFER TO MULTIPLE MEMBER TO MEMBER LVL/LSL (Flush) READ ALL NOTES ON THIS PAGE AND ON CONNECTION DETAIL FOR PLY TO PLY Label Description Width Depth ENGINEERING NOTE PAGE ENP-2. THIS NAILING OR BOLTING REQUIREMENTS. F18 Forex NOTE PAGE IS AN INTEGRAL PART OF THIS N.A. EL-MASRI 2.0E-3000Fb LVL PASS THRU FRAMING SQUASH CALCULATION SUMMARY PAGE AS IT F6 BLOCK IS REQUIRED AT ALL par trasp Forey CONTAINS SPECIFICATIONS AND CRITERIA 2.0E-3000Fb LVL POINT LOADS OVER BEARINGS. USED IN THE DESIGN OF THIS COMPONENT. F5 огех Aug 13, 2018 2.0E-3000Fb LVL F4 2.0E-3000Fb LVL Joist (Flush) Label Description Width | Depth | Qty | Plies | F6-A - 2 ply J9 NJ60U JB NJH J5 NJH J4 NJH J3 NJH J2 NJH F6-B - 2 ply J1 NJH J3-J J6 NJH Rim Board Label Description Width Depth F4-A - 2 ply Norbord Rimboa 1.125 11.875 ? DROPPED BBO Plus 1,125 X 11.875 langer Pcs Description Label H1 2 Unknown =,J5-E≤ Hanger H2 HUC410 (Min) F6-D - 2 ply НЗ 1 HGUS410 H5 28 LT251188 Blocking Label Description Width Depth BLK1 NJH NOTES: ■J3-A= F18-A - 1 ply . Framer to verify dimensions on the architectural drawings. 2. Double joist only require filler/backer ply when supporting another member using a face-mounted hanger.

3. Install 2x4 blocking @ 24"o/c under parallel non-load bearing walls. 4. Install single-ply flush window header along inside face of Refer to Nascor specifier guide for installation works. 6. Squash blocks recommended to be installed at end bearing on all first level joists which support loading from above exceeding two levels floor or roof 7. Load transfer blocks to be installed under all point loads. 3. It shall be the frame's responsibility that floor joists and beams are fastened as per the hanger manufacturer's standards. **■J5-O**■ Refer to Multiple Member Connection Detail to ply to ply nailing or bolting requirements Rim parallel to joists: 1-1/8" rimboard with 2"x4" block (1/16" longer than rim depth @ 16"o/c). All other components and structural elements J4-C supporting the floor system such as beams, walls, columns, and foundation walls, and footings including anchorage of components and bracing for lateral stability are the responsibility of Others. واصلا الأحاج الأحراج ال Hatch are represents ceramic tiled floor with an additional dead load =J5-K= The framing shown on this layout may deviate from the architectural R1 2 prior to construction أمواهم محافرهم ومحاملات فالحداد R1 THIS CERTIFICATION IS TO CONFIRM THAT: Legend REGION DESIGN INC. I. THE LOADS USED IN THE CALCULATION OF THE ATTACHED Point Load Support APPROVED COMPONENTS CONFORM TO THE FLOOR ASSEMBLY SHOWN ON THIS LAYOUT. 0 Load from Above Date: Rev. 1, 5//2018 Project No: 3008 Wall 2. THE FLOOR JOISTS COMPLY WITH THE NASCOR SPAN TABLE Wall Opening Norbord Rimboard Plus 1.125 X 11.875 THE FLOOR SYSTEM MUST BE ASSEMBLED IN ACCORDANCE TO NJ 11.875 THE NASCOR SPECIFIER GUIDE, MULTI-PLY MEMBERS MUST BE ATTACHED TOGETHER AS PER THE INCLUDED MULTIPLE NJ40U 11.875 MEMBER CONNECTION DETAIL. NJ60U 11.875 ALL OTHER COMPONENTS AND STRUCTURAL ELEMENTS NJH 11.875

**PAGE 25 OF 35** 

Pcs Length

12-0-0 1 Layout Name 10-0-0 8 8-0-0 LSD 6-0-0

16

28

9

4-0-0

Supported

Member

fasteners

6 10d

16 16d

Pcs Length

Varies 43-0-0

4

Qty Plies

Qty Plies

Beam/Girder

fasteners

14 16d

46 16d

Qty | Plies |

4 10dx1 1/2 2 10dx1 1/2

1.75

1.75

1.75

1.75

11 875

11.875

11.875

11.875

3.5 11.875

2.5 11.875

2.5 11.875

2.5 11.875

2.5 11.875

2.5 11.875

2.5 11.875

2.5 11.875

Skew Slope

2.5 11.875 LinFt

**GRANDBROOKE 12-ELEV 2** Design Method Description

Created Pcs Length June 25, 2018 19 18-0-0 Builder 16-0-0 GREENPARK

14-0-0 15 12-0-0 Sales Rep RM 10-0-0 8-0-0 Designer 6-0-0 RO

Shipping

Canada

Project Pcs Length **Builder's Project** 

> **Kott Lumber Company** 14 Anderson Blvd Stouffville, Ontario

K2H7V1 905-642-4400

Job Path

C:\Users\royochavillo\Desktop **IGREENPARK-MINNISALE HOMES-**MODEL-GRANDBROOKE 12-FLOORS **\ELEV 2\GRANDBROOKE 12-ELEV** 

Second Floor

Design Method LSD Building Code NBCC 2010 / OBC 2012

Floor Loads Live

Dead Deflection Joist

40

15

480

360

480

360

360

240

480

240

Nailed & Glued

Gypsum 1/2"

LL Span L/ TL Span L/

LL Cant 2L/ TL Cant 2L/

Deflection Girder LL Span L/

TL Span L/ LL Cant 2L/ TL Cant 2L/

Decking Deck SPF Plywood Thickness

Fastener Vibration Ceiling

and structural drawings. Project Engineer to review and apporve the deviation

**ARCHITECTURAL DRAWINGS:** 

8700 Dufferin St., Concord Model: Grandbrooke 12, Elevation 2

- 2. Nascor CCMC 13535-R
- 3. LVL CCMC -14056-R
- 4. CAN/CSA-086-09

Forex 2.0E-3000Fb LVL 1.75 X 11.875

1. OBC 2012 O.Reg 332/12 as amended

- 5. CCMC -12787-R APA PR-L310(C)

Version 18.40.105 Powered by iStruct™

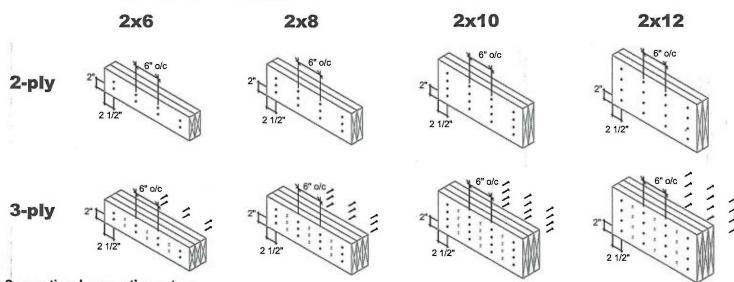
This layout is to be used as an installation guide only. It is meant to be used in conjunction with the architectural and structural drawings, not to replace them

SUPPORTING THE FLOOR SYSTEM SUCH AS BEAMS, WALLS.

COLUMNS AND FOUNDATION WALLS AND FOOTINGS
INCLUDING ANCHORAGE OF COMPONENTS AND BRACING FOR LATERAL STABILITY ARE THE RESPONSIBILITY OF OTHERS.

# MULTIPLE MEMBER CONNECTIONS

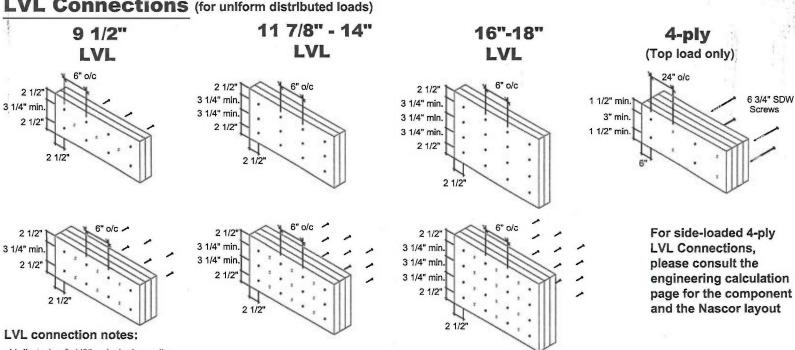
# Conventional Connections (for uniform distributed loads)



### Conventional connection notes:

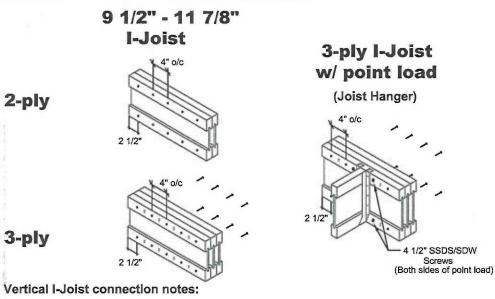
- -Nails to be 3" 10d spiral wire nails.
- -Nails to be located a minimum of 2" from the top and bottom of the member. Start all nails a minimum of 2 1/2" in from ends.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail driven from the opposite side.

## LVL Connections (for uniform distributed loads)



- -Nails to be 3 1/2" spiral wire nails.
- -Nails to be located a minimum of 2 1/2" from the top and bottom of the member. Start all nails a minimum of 2 1/2" in from ends.
- -Minimum 3 1/4" spacing between rows.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail or screw driven from the opposite side.

# Vertical I-Joist Connections (for uniform distributed loads)



- -Nails to be 3" spiral wire nails.
- -Nails to be located at centre of top and bottom flanges. Start all nails a minimum of 2 1/2" in from ends.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail driven from the opposite side.

**MULTI-PLY** CONNECTION **DETAILS** 

> Date: November 30, 2016 Scale: NTS

KOTT 3228 Moodie Drive Ottawa, ON **K2H 7V1** Ph: 613-838-2775 Fx: 613-838-4751

### **Engineering Note Page (ENP-2)**

REVISION 2009-10-09

### Please read all notes prior to installation of the component

### **DESIGN INFORMATION**

This building component is certified as an individual component for the loads and conditions shown on the calculation and drawing page.

The responsibility of the undersigned engineer is <u>only</u> limited to the calculation of this building component for the loads and conditions shown on this drawing.

The responsibility of the undersigned is limited to the verification of the structural capacity of the NASCOR floor joists and LVL beams based on placement as shown on the layout. The loads applied are limited to the gravity effects of the specified loads. The structural integrity of the building and the effect of wind, uplift, seismic, lateral or other forces, calculation of adequate support and anchorage of components, as well as the dimensions and design loads used to calculate components are the responsibility of the overall building designer.

Floor joists and OSB rim board are designed to carry uniformly distributed loads only. Point loads should be transferred through the floor cavity with squash blocks. Structural elements such as walls, posts, connectors, and squash blocks are the responsibility of the overall building designer.

The undersigned engineer disclaims any responsibility for damages as a result of being furnished faulty or incorrect information, specifications and/or designs.

Installation of NASCOR joists is to be carried out in accordance with the current edition of the manufacturer's approved literature available at <a href="http://www.nascor.ca">http://www.nascor.ca</a>.

### CODE

This building component is designed in accordance with the National Building Code of Canada, the Ontario Building Code, CCMC and Canadian Standards Association guidelines.

### **COMPONENT**

- 1. The building component used in construction must be the same as indicated on the drawings.
- 2. The building component must be installed and assembled as per specification shown on the drawing and in accordance with the manufacturer's assembly and installation.
- 3. Members consisting of multiple plies must be connected as per the document "Multi-ply Connection Details".
- 4. Pass-thru squash block framing is required at all point loads over bearings.

### HANDLING AND INSTALLATION

Do not drill any hole, cut or notch a certified building component without a written preauthorization.



# MULTIPLE MEMBER CONNECTIONS

### Conventional Connections (for uniform distributed loads)

2x6

2x8

2x12

2-ply







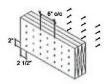


3-ply









### Conventional connection notes:

-Nails to be 3" 10d spiral wire nails.

-Nails to be located a minimum of 2" from the top and bottom of the member. Start all nails a minimum of 2 1/2" in from ends. -Number of rows and spacing as per details shown, unless noted otherwise
 -'X' represents nail driven from the opposite side.

11 7/8" - 14"

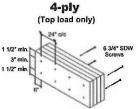
### LVL Connections (for uniform distributed loads)

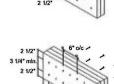
LVL

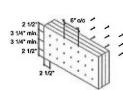


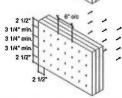
16"-18" LVL

3 1/4" mir









For side-loaded 4-ply LVL Connections, please consult the engineering calculation page for the component and the Nascor layout

### LVL connection notes:

-Nails to be 3 1/2" spiral wire nails.

- -reals to be 3 µ2 sprai wire hears.

  Alalis to be located a minimum of 2 1/2\* from the top and bottom of the member. Start all halls a minimum of 2 1/2\* in from ends.

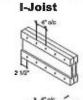
  -Minimum 3 1/4\* spacing between rows.

  -Number of rows and spacing as per details shown, unless noted otherwise.

  -"X" represents hall or screw driven from the opposite side.

### Vertical I-Joist Connections (for uniform distributed loads)

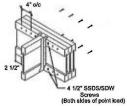
9 1/2" - 11 7/8"



3-ply I-Joist w/ point load

(Joist Hanger)





3-ply

2-ply

Vertical I-Joist connection notes:

- -Nails to be 3" spiral wire nails
- -Nalls to be located at centre of top and bottom flanges. Start all nalls a minimum of 2 1/2" in from ends.
- -Number of rows and spacing as per details shown
   "X" represents nail driven from the opposite side.

MULTI -PLY CONNECTION DETAILS

3228 Moodle Drive Ottawa, ON K2H 7V1 Ph: 613-838-2775 Fx: 613-838-4751

Page 1 of 2



Address:

**GREENPARK** Project:

8/13/2018

Designer: RO

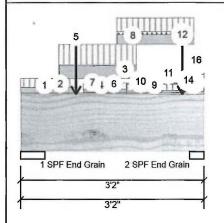
Job Name: GRANDBROOKE 12-ELEV 1-LOB OPT.

Project #:

Forex 2.0E-3000Fb LVL FH6-A

1.750" X 11.875"

2-Ply - PASSED Level: Ground Floor



REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS.



Member In	formation						-
Type: Plies: Moisture Con	Girder 2 dition: Dry		Applicat Design Building	Method: L	Floor (Resident .SD NBCC 2010 / O		
Deflection LL Deflection TL Importance: General Load Floor Live:	: 240 Normal		Load Sh Deck: Vibration	١	lo lot Checked lot Checked		
Dead:	15 PSF						
Analysis Re	sults						٦
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case	٦
Moment	1848 ft-lb	1'7 11/16"	34261 ft-lb	0.054 (5%)	1.25D+1.5L +0.5S	L	-
Unbraced	1848 ft-lb	1'7 11/16"	34261 ft-lb	0.054 (5%)	1.25D+1.5L	L	

**Unfactored Reactions UNPATTERNED Ib (Uplift)** R

srg	Live	Dead	Snow	vvind
1	1195	855	78	0
2	1074	868	237	0

**Bearings and Factored Reactions** 

Bearing	Length	Сар.	React D/L lb	Total	Ld, Case	Ld. Comb.
1 - SPF End Grain	5.000"	22%	1068 / 1831	2900	L	1.25D+1.5L +0.5S
2 - SPF End	3.000"	36%	1085 / 1729	2814	L	1.25D+1.5L +0.5S

+0.58 0.195 (20%) 1.25D+1.5L L +0.58

Grain

Perm Defl in. 0.002 1'7 13/16" 0.088 (L/360) 0.030 (3%) D (L/14367)

1'11 7/8" 11596 lb

17 3/4" 0.088 (L/360) 0.040 (4%) L+0.5S L 1'7 3/4" 0.131 (L/240) 0.040 (4%) D+L+0.5S L

Uniform

READ ALL NOTES ON THIS PAGE AND ON

**ENGINEERING NOTE PAGE ENP-2. THIS** 

NOTE PAGE IS AN INTEGRAL PART OF THIS **CALCULATION SUMMARY PAGE AS IT** CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



Design Notes

Shear

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.

Part, Uniform

- 5 Top braced at bearings.
- 6 Bottom braced at bearings.

LL Defl inch 0.003 (L/9858)

TL Defl inch 0.005 (L/5847)

2264 lb

7 Lateral slenderness ratio based on full section width. ID Trib Width Wind Load Type Location Side Dead Live Snow 0-0-0 to 0-9-14 40 PLF 0 PLF 0 PLF 0 PLF Part. Uniform Top 1 2 Part. Uniform 0-0-0 to 0-11-14 Near Face 154 PLF 263 PLF 0 PLF 0 PLF

Near Face

Continued on page 2...

Notes

3

Calculated Shuctured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

0-7-14 to 1-11-14

Handling & Installation

LVL beams must not be cut or drilled
 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code

approvals

Damaged Bearns must not be used

Design assumes top edge is laterally restrained

Provide lateral support at bearing points to avoid
lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

364 PLF

Manufacturer Info

575 PLF

APA: PR-L318

22 PLF

Kott Lumber Company 14 Anderson Blvd, Ontario K2H7V1 905-642-4400

Comments

J7

J7

Wall Self Weight



0 PLF



Client:

Address:

GREENPARK Project:

Page 2 of 2

Designer: RO

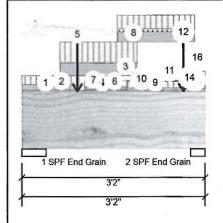
Job Name: GRANDBROOKE 12-ELEV 1-LOB OPT.

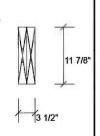
Project #:

Forex 2.0E-3000Fb LVL FH6-A

1.750" X 11.875"

2-Ply - PASSED Level: Ground Floor





Continued	from page 1								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
4	Part. Uniform	0-9-14 to 1-9-14		Тор	40 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
5	Point	0-11-8		Тор	199 lb	283 lb	29 lb	0 lb	Header Column
6	Part. Uniform	1-1-0 to 1-9-14		Тор	40 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
7	Part. Uniform	1-1-0 to 1-9-14		Тор	67 PLF	179 PLF	0 PLF	0 PLF	J8
8	Part. Uniform	1-7-14 to 2-11-14		Near Face	281 PLF	391 PLF	86 PLF	0 PLF	J7
9	Part. Uniform	1-9-14 to 2-9-11		Тор	40 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
10	Part. Uniform	1-9-14 to 2-9-11		Тор	67 PLF	179 PLF	0 PLF	0 PLF	J8
11	Part. Uniform	1-9-14 to 2-9-11		Тор	40 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
12	Point	2-9-8		Тор	67 lb	0 lb	115 lb	0 lb	Header Column
13	Part. Uniform	2-9-11 to 3-2-0		Тор	80 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
14	Part. Uniform	2-9-11 to 3-2-0		Тор	134 PLF	358 PLF	0 PLF	0 PLF	J8
15	Part. Uniform	2-9-11 to 3-2-0		Тор	80 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
16	Part. Uniform	2-11-0 to 3-2-0		Тор	46 PLF	0 PLF	106 PLF	0 PLF	
	Self Weight				10 PLF				

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.

### Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the Intended application, and to verify the dimensions and loads.

### Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

Handling & Installation

1. UV beams must not be cut or drilled

2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

3. Damaged Beams must not be used

4. Design assumes top edge is laterally restrained

5. Provide lateral support at bearing points to avoid lateral dispracement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA; PR-L318

Kott Lumber Company 14 Anderson Blvd, Ontario Canada K2H7V1 905-642-4400



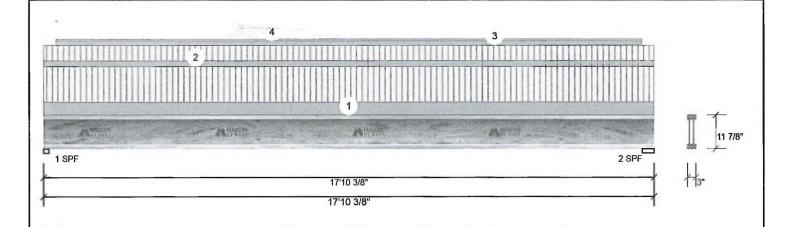
isDesign™

Address:

Job Name: GRANDBROOKE 12-ELEV 1

Project #:

11.875" 2-Ply - PASSED Level: Ground Floor



. 31				19				
Plies:	2	Design Method:	LSD	1	420	209	0	0
Moisture Condition	n: Dry	Building Code:	NBCC 2010 / OBC 2012	2	429	213	0	0
Deflection LL:	360	Load Sharing:	No					
Deflection TL:	240	Deck:	Not Checked					
Importance:	Normal	Vibration:	Not Checked					
General Load								
Floor Live:	40 PSF			Bearings a	ind Factore	d Reactions		
Dead:	15 PSF			Bearing L	ength C	ap. React D/L lb	Total Ld. Case	Ld. Comb.
				1 - SPF 1.	.875" 3	3% 261 / 631	891 L	1.25D+1.5L
				2-SPF 4	125" 2	7% 266 / 644	910 L	1.25D+1.5L

Bra

Floor (Residential)

Ana	lysis	Resul	ts

Member Information

Girder

Type:

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	3868 ft-lb	8'10 1/16"	9020 ft-lb	0.429 (43%)	1.25D+1.5L	L
Unbraced	3868 ft-lb	8'10 1/16"	3903 ft-lb	0.991 (99%)	1.25D+1.5L	L
Shear	884 lb	17'7"	3400 lb	0.260 (26%)	1.25D+1.5L	L
Perm Defl in.	0.108 (L/1941)	8'10 1/8"	0.583 (L/360)	0.190 (19%)	D	Uniform
LL Defl inch	0.216 (L/973)	8'10 1/8"	0.583 (L/360)	0.370 (37%)	L	L
TL Defl inch	0.324 (L/648)	8'10 1/8"	0.874 (L/240)	0.370 (37%)	D+L	L

Application:

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

Unfactored Reactions UNPATTERNED Ib (Uplift)

Dead

Snow

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.

Live

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS **CALCULATION SUMMARY PAGE AS IT** CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



Wind

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 3'8" o.c.
- 5 Bottom flange braced at bearings.

Ī	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
	1	Tie-In	0-0-0 to 17-10-6	(Span)1-7-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	2	Tie-In	0-0-0 to 17-10-6	(Span)0-8-10	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	3	Part. Uniform	0-4-6 to 17-6-2		Тор	4 PLF	0 PLF	0 PLF	0 PLF	
	4	Part. Uniform	0-4-6 to 17-6-2		Тор	2 PLF	0 PLF	0 PLF	0 PLF	

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suifability of the inlended application, and to verify the dimensions and loads.

### Lumber

Dry service conditions, unless noted otherwise
 Unist not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- Handling & Installation

  1. Joist flanges must not be cut or drilled

  2. Refer to latest copy of the Lioist product information details for framing details, stiffener tables, web hole chart. brdging details, multi-pyl testening details and handling/erection details

  3. Damaged bolists must not be used

  4. Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeness for point load as shown Minimum point load bearing length>= 3.5 inches
 To for flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Nascor by Kott

Kott Lumber Company 14 Anderson Blvd, Ontario Canada K2H7V1





Page 1 of 2



Client:

Address:

Project:

GREENPARK

Date:

8/10/2018 Designer: RO

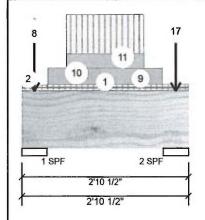
Job Name: GRANDBROOKE 12-ELEV 1

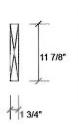
Project #:

Forex 2.0E-3000Fb LVL

1.750" X 11.875" - PASSED

Level: Ground Floor





MEHIDEI IIIIOI	mation	office to the detailed in the little of the territory								
Туре:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snov	v	Wind
Plies:	1	Design Method:	LSD	1	221		200		0	0
Moisture Conditio	n: Dry	Building Code:	NBCC 2010 / OBC 2012	2	200		195		O	0
Deflection LL:	360	Load Sharing:	No							
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal	Vibration:	Not Checked							
General Load				-						
Floor Live:	40 PSF	4		Bearings	and Fact	tored Re	eactions			
Dead:	15 PSF			Bearing L	ength	Cap. F	React D/L lb	Total	Ld. Case	Ld. Comb.
				1-SPF 5	5.250"	10%	250 / 331	581	L	1.25D+1.5L
				2-SPF 5	5.250"	10%	243 / 301	544	L	1.25D+1.5L
malaria Dane	4			t-						

Analysis Results

Member Information

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	271 ft-lb	1'5 1/4"	17130 ft-lb	0.016 (2%)	1.25D+1.5L	L
Unbraced	271 ft-lb	1'5 1/4"	15450 ft-lb	0.018 (2%)	1.25D+1.5L	L
Shear	39 lb	1'4 3/8"	5798 lb	0.007 (1%)	1.25D+1.5L	L
Perm Defl in.	0.001 (L/40207)	1'5 5/16"	0.071 (L/360)	0.010 (1%)	D	Uniform
LL Defl inch	0.001 (L/33110)	1'5 5/16"	0.071 (L/360)	0.010 (1%)	L	L
TL Defl inch	0.001 (L/18157)	1'5 5/16"	0.106 (L/240)	0.010 (1%)	D+L	L

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

Unfactored Reactions UNPATTERNED Ib (Unlift)

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS

READ ALL NOTES ON THIS PAGE AND ON **ENGINEERING NOTE PAGE ENP-2. THIS** NOTE PAGE IS AN INTEGRAL PART OF THIS **CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA** USED IN THE DESIGN OF THIS COMPONENT.



**Design Notes** 

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.

4 BURUITI D	naceu at bearings.								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 2-10-8	(Span)0-6-14	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 0-5-4	(Span)0-5-2	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	0-2-10		Тор	1 lb	3 lb	0 lb	0 lb	J8
4	Point	0-2-10		Тор	3 lb	8 lb	0 lb	0 lb	J5
5	Point	0-2-10		Тор	6 lb	0 lb	0 lb	0 lb	Wall Self Weight

Top

Continued on page 2...

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the Intended application, and to verify the dimensions and loads.

Point

### Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

0 - 2 - 10

- Handling & Installation

  1. IVL beams must not be cut or drilled

  2. Refer to manufacturer's product information regarding installation requirements, muldi-ply fastening details, beam strength values, and code approvals

  3. Damaged Beams must not be used

  4. Design assumes top edge is laterally restrained

  5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent

5 lb

14 lb

Manufacturer Info Forex

APA: PR-L318

0 lb

0 lb J8

Kott Lumber Company 14 Anderson Blvd, Onlario 905-642-4400



Page 2 of 2



Client:

Project:

GREENPARK

Address:

Date: 8/10/2018

RO Designer:

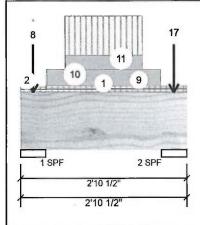
Job Name: GRANDBROOKE 12-ELEV 1

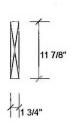
Project #:

Forex 2.0E-3000Fb LVL

1.750" X 11.875" - PASSED

Level: Ground Floor





Continued	from page 1								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
7	Point	0-2-10		Тор	14 lb	34 lb	0 lb	0 lb	J5
8	Point	0-2-10		Тор	24 lb	0 lb	0 lb	0 lb	Wall Self Weight
9	Tie-In	0-5-4 to 2-10-8	(Span)0-7-10	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
10	Part. Uniform	0-5-6 to 2-5-2		Тор	80 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
11	Part. Uniform	0-9-4 to 2-1-4		Тор	73 PLF	193 PLF	0 PLF	0 PLF	J8
12	Point	2-7-14		Тор	5 lb	14 lb	0 lb	0 lb	J8
13	Point	2-7-14		Тор	3 lb	8 lb	0 lb	0 lb	J4
14	Point	2-7-14		Тор	24 lb	0 lb	0 lb	0 lb	Wall Self Weight
15	Point	2-7-14		Тор	4 lb	10 lb	0 lb	0 lb	J4
16	Point	2-7-14		Тор	2 lb	5 lb	0 lb	0 lb	J8
17	Point	2-7-14		Тор	9 lb	0 lb	0 lb	0 lb	Wall Self Weight
	Self Weight				5 PLF				

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS.

### Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the inlended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

### chemicals

Handling & Installation

- Handling & Installation

  1. LVL beams must not be cut or drilled

  2. Reifer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

  3. Damaged Beams must not be used

  4. Design assumes top edge is laterally restrained

  5. Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex

APA: PR-L318

Kott Lumber Company 14 Anderson Blvd, Ontario Canada K2H7V1 905-642-4400



### NE0818-059 GREENPARK-MINNISALE HOMES-GRANDBROOKE 12-ELEV 1 & 2

isDesign™

Client:

Address:

GREENPARK

Project:

Date:

Designer:

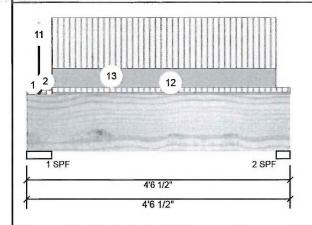
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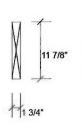
Project #:

F3-A Forex 2.0E-3000Fb LVL

1.750" X 11.875" - PASSED

Level: Ground Floor





**PAGE 10 OF 35** 

Page 1 of 2

Member Info	rmation		
Type:	Girder	Application:	Floor (Residential)
Plies:	1	Design Method:	LSD
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF	ı	

**Unfactored Reactions UNPATTERNED Ib (Uplift)** 

Brg	Live	Dead	Snow	vvina
1	590	259	0	0
2	448	179	0	0

### **Bearings and Factored Reactions**

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	5.250"	21%	324 / 885	1209	L	1.25D+1.5L
2 - SPF	2.875"	29%	224 / 672	896	L	1.25D+1.5L

**Analysis Results** 

15 PSF

Dead:

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	911 ft-lb	2'4 7/16"	17130 ft-lb	0.053 (5%)	1.25D+1.5L	L
Unbraced	911 ft-lb	2'4 7/16"	11270 ft-lb	0.081 (8%)	1.25D+1.5L	L
Shear	461 lb	1'4 3/8"	5798 lb	0.079 (8%)	1.25D+1.5L	L
Perm Defl in.	0.002 (L/23175)	2'4 1/2"	0.133 (L/360)	0.020 (2%)	D	Uniform
LL Defl inch	0.005 (L/9197)	2'4 1/2"	0.133 (L/360)	0.040 (4%)	L	L
TL Defl inch	0.007 (L/6584)	2'4 1/2"	0.199 (L/240)	0.040 (4%)	D+L	L

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA



**Design Notes** 

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.
- 4 Bottom braced at bearings

USED IN THE DESIGN OF THIS COMPONENT.

4 DOLLOIT DI	raced at bearings.								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 0-4-2	(Span)0-6-12	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 0-5-4	(Span)0-5-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	0-2-10		Тор	10 lb	26 lb	0 lb	0 lb	J8
4	Point	0-2-10		Тор	2 lb	5 lb	0 lb	0 lb	J1
5	Point	0-2-10		Тор	6 lb	0 lb	0 lb	0 lb	Wall Self Weight
6	Point	0-2-10		Тор	19 lb	52 lb	0 lb	0 lb	J8
7	Point	0-2-10		Тор	4 lb	11 lb	0 lb	0 lb	J1
Continued on p	page 2								

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the Inlended application, and to verify the dimensions and loads.

### Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or con-

Handling & Installation

- Handling & Installation

  1. LVL beams must not be cut or drilled

  2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

  3. Damaged Beams must not be used

  4. Design assumes top edge is laterally restrained

  5. Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent coording

This design is valid until 7/10/2021

Manufacturer Info APA: PR-L318

Kott Lumber Company 14 Anderson Blvd, Ontario Canada K2H7V1 905-642-4400



Page 2 of 2



Client:

Address:

GREENPARK Project:

Date:

RO

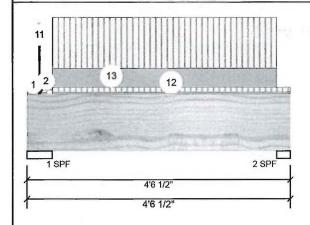
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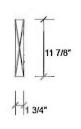
Project #:

Designer:

1.750" X 11.875" - PASSED Forex 2.0E-3000Fb LVL

Level: Ground Floor





Continued from	om page 1								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
8	Point	0-2-10		Тор	12 lb	0 lb	0 lb	0 lb	Wall Self Weight
9	Point	0-2-10		Тор	14 lb	37 lb	0 lb	0 lb	J8
10	Point	0-2-10		Тор	3 lb	8 lb	0 lb	0 lb	J1
11	Point	0-2-10		Тор	8 lb	0 lb	0 lb	0 lb	Wall Self Weight
12	Tie-In	0-5-4 to 4-6-8	(Span) 0-11-12	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
13	Part. Uniform	0-5-4 to 4-3-10		Тор	<b>79 PLF</b>	210 PLF	0 PLF	0 PLF	
W. C. C.	Self Weight				5 PLF				

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS.

Calculated Shuchared Designs is responsible only of the shucharal adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

### Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

Handling & Installation

1. IVL beams must not be cut or drilled

2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastering details, beam strength values, and code approvals

3. Damaged Beams must not be used

4. Design assumes top edge is laterary restrained

5. Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex APA: PR-L318

Kott Lumber Company 14 Anderson Blvd, Ontario K2H7V1 905-642-4400





Client:

**GREENPARK** 

8/10/2018

Page 1 of 1

Project: Address: Designer: RO

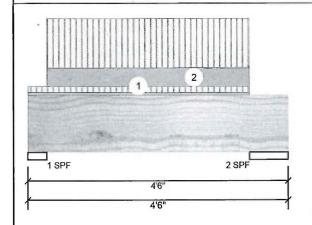
Job Name: GRANDBROOKE 12-ELEV 1

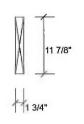
Project #:

Forex 2.0E-3000Fb LVL

1.750" X 11.875" - PASSED

Level: Ground Floor





mation			Unfacto	red Reac	tions U	NPATTERN	ED lb (	(Uplift)	
Girder	Application:	Floor (Residential)	Brg	Live		Dead	Sno	w	Wind
1	Design Method:	LSD	1	362		146		0	0
n: Dry	Building Code:	NBCC 2010 / OBC 2012	2	354		145		0	0
360	Load Sharing:	No							
240	Deck:	Not Checked							
Normal	Vibration:	Not Checked							
40 PSF			Bearings	and Fac	tored	Reactions			
15 PSF			Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.
			1 - SPF	4.000"	17%	183 / 542	725	L	1.25D+1.5L
			2-SPF	8.000"	8%	181 / 531	713	L	1.25D+1.5L
	Girder 1 nn: Dry 360 240 Normal	Girder Application: 1 Design Method: bit: Dry Building Code: 360 Load Sharing: 240 Deck: Normal Vibration:	Girder Application: Floor (Residential)  1 Design Method: LSD Building Code: NBCC 2010 / OBC 2012 360 Load Sharing: No 240 Deck: Not Checked Normal Vibration: Not Checked	Application: Floor (Residential)   Brg   1   Design Method: LSD   1   2   2   360   240   Deck: Not Checked   Normal   Vibration: Not Checked   Bearings   15 PSF   Bearing   1 - SPF	Application: Floor (Residential)   Brg   Live	Application: Floor (Residential)   Brg   Live	Application:   Floor (Residential)   Brg   Live   Dead	Application: Floor (Residential)   Brg   Live   Dead   Snor	Application: Floor (Residential)   Brg   Live   Dead   Snow

**Analysis Results** 

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	664 ft-lb	2'1"	17130 ft-lb	0.039 (4%)	1.25D+1.5L	L
Unbraced	664 ft-lb	2'1"	12240 ft-lb	0.054 (5%)	1.25D+1.5L	L
Shear	333 lb	2'10 7/8"	5798 lb	0.057 (6%)	1.25D+1.5L	L
Perm Defl in.	0.001 (L/31458)	2'1 1/16"	0.121 (L/360)	0.010 (1%)	D	Uniform
LL Defl inch	0.003 (L/12616)	2'1 1/16"	0.121 (L/360)	0.030 (3%)	L	L
TL Defl inch	0.005 (L/9004)	2'1 1/16"	0.181 (L/240)	0.030 (3%)	D+L	L

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS.

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



**Design Notes** 

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Top braced at bearings.
- 3 Bottom braced at bearings

3 DOMONI L	nacea at bearings.								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 3-10-0	(Span)1-1-7	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Part. Uniform	0-4-0 to 3-10-0		Тор	68 PLF	180 PLF	0 PLF	0 PLF	
	Self Weight				5 PLF				

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the Inlended application, and to verify the dimensions and loads. Lumber

chemicals

Handling & Installation

- Handling & Installation

  1. IVJ beams must not be cut or drilled

  2. Refer to manufacturer's product information regarding installation requirements, mutti-ply fastening details, beam strength values, and code approvals

  3. Damaged Beams must not be used

  4. Design assumes top edge is laterally restrained

  5. Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

This design is valid until 7/10/2021

Manufacturer Info APA: PR-L318

Canada K2H7V1

Kott Lumber Company 14 Anderson Blvd, Ontario



Page 1 of 1



isDesign"

GREENPARK Client:

Project: Address: Date: 8/10/2018

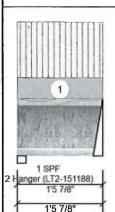
Designer: RO

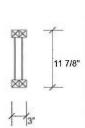
Job Name: GRANDBROOKE 12-ELEV 1

Project #:

2-Ply - PASSED 11.875" NJ

Level: Ground Floor





Vlember Inf	ormation			-		Unfacto	red Reac	tions (	INPATTERN	ED lb	(Uplift)	
Туре:	Girder	**	Application	on: F	foor (Residential)	Brg	Live		Dead	Sno	w	Wind
Plies:	2		Design M	lethod: L	.SD	1	48		18		0	0
Moisture Cond	ition: Dry		Building (	Code: N	BCC 2010 / OBC 20	12 2	49		18		0	0
Deflection LL:	360		Load Sha	aring: N	lo .							
Deflection TL:	240		Deck:	N	lot Checked							
Importance:	Normal		Vibration:		lot Checked							
General Load												
Floor Live:	40 PSF					Bearing	s and Fac	tored	Reactions			
Dead:	15 PSF					Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
						1 - SPF	1.875"	4%	23 / 72	95	L	1.25D+1.5L
						2-	2.000"	4%	23 / 73	96	L	1.25D+1.5L
Analysis Re	ults					Hanger						
Analysis	Actual	Location	Allowed	Capacity	Comb. Cas	e REEER	O MIII TIP	I E MEN	BER TO MEM	BER		
Moment	27 ft-lb	8 7/8"	9020 ft-lb	0.003 (0%)	1.25D+1.5L L				R PLY TO PLY	DEIX		
Unbraced	27 ft-lb	8 7/8"	8539 ft-lb	0.003 (0%)	1.25D+1.5L L	NAILING	OR BOLTI	NG REG	QUIREMENTS.			
Shear	83 lb	1 1/8"	3400 lb	0.024 (2%)	1.25D+1.5L L		IRU FRAM				a0F	ESSIONAL
Perm Defl in.	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)			IS REQUIR				100	181
LL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		IPOINT L	OADS OVE	K BEAL	KINGS.		186	
TL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)					PAGE AND O	N	S N.A.	EL-MASRI S
Design Not	es								E ENP-2. THIS AL PART OF T	HIS (	Xe toa	FFASON

1 Fill all hanger nailing holes.

5 Top flange braced at bearings.

2 Girders are designed to be supported on the bottom edge only.

4 Top loads must be supported equally by all plies.

3 Multiple plies must be fastened together as per manufacturer's details.

6 Bottom flange braced at bearings. ID Wind Load Type Trib Width Side Dead Live Comments Location Snow 15 PSF 40 PSF 0 PSF 0 PSF 1 Tie-In 0-0-0 to 1-5-14 (Span)3-3-0 Top



Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

### Lumber

Dry service conditions, unless noted otherwise
 Upost not to be treated with fire relardant or corrosive

chemicals

- Handling & Installation

  1. IJoist flanges must not be cut or drilled

  2. Refer to latest copy of the IJoist product Information details for framing details, stiffener tables, web hole chart, bridging details, multi-ply fastering details and handling/erection details

  3. Damaged IJoist must not be used

  4. Design assumes top flange to be laterally restrained by stlached sheathing or as specified in engineering notes.

Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 For flat roofs provide proper drainage to prevent ponding

This design is valid until 7/10/2021



Nascor by Kott

**CALCULATION SUMMARY PAGE AS IT** 

CONTAINS SPECIFICATIONS AND CRITERIA

USED IN THE DESIGN OF THIS COMPONENT.

Kott Lumber Company 14 Anderson Blvd, Ontario Canada K2H7V1 905-642-4400





Aug 13, 2018

**GREENPARK** 

Page 1 of 1



isDesign"

Project:

Address:

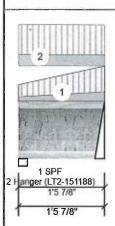
Designer: RO

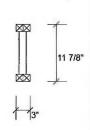
Job Name: GRANDBROOKE 12-ELEV 1

Project #:

2-Ply - PASSED 11.875" NJ

Level: Ground Floor





Member Info	rmation			Unfacto	red Reac	tions U	NPATTERN	ED lb (	(Uplift)	
Type:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Sno	w	Wind
Plies:	2	Design Method:	LSD	1	74		28		0	0
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	84		31		0	0
Deflection LL:	360	Load Sharing:	No	100						
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal	Vibration:	Not Checked							
General Load										
Floor Live:	40 PSF			Bearing	s and Fac	tored	Reactions			
Dead:	15 PSF			Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
				1 - SPF	1.875"	5%	35 / 112	146	L	1.25D+1.5L
				2-	2.000"	6%	39 / 126	165	L	1.25D+1.5L

Hanger

Analysis Re	sults					
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	44 ft-lb	9 1/16"	9020 ft-lb	0.005 (0%)	1.25D+1.5L	L
Unbraced	44 ft-lb	9 1/16"	8539 ft-lb	0.005 (1%)	1.25D+1.5L	L
Shear	140 lb	1'4 5/8"	3400 lb	0.041 (4%)	1.25D+1.5L	L
Perm Defl in.	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
LL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		

0 999.000 (L/0) 0.000 (0%)

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS.

READ ALL NOTES ON THIS PAGE AND ON **ENGINEERING NOTE PAGE ENP-2. THIS** NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



### **Design Notes**

1 Fill all hanger nailing holes.

TL Defl inch 0.000 (L/999)

- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top flange braced at bearings.
- 6 Bottom flange braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-5-14	(Span)1-3-12 to 2-9-10	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 1-5-14	(Span)3-3-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

### Notes

Calculated Struchared Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the landed application, and to verify the dimensions and loads.

### Lumber

Dry service conditions, unless noted otherwise
 Lioist not to be treated with fire retardant or corrosive

- Handling & Installation

  1. Jiost flanges must not be cut or drilled
  2. Refer to latest copy of the Liost product information detaits for framing details, stiffener tables, web note chart, bridging details, multi-py fastening details and handling/ferection details
  3. Damaged Lioist must not be used
  4. Design assumes top flange to be laterally restrained by attached shealthing or as specified in engineering notes.

Manufacturer Info Nascor by Kott

Kott Lumber Company 14 Anderson Blvd, Ontario K2H7V1 905-642-4400





### Notes

Continued on page 2...

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the inlended Lumber

Dry service conditions, unless noted otherwise
 Uplist not to be treated with fire retardant or corrosive

Handling & Installation

Noist flanges must not be cut or drilled
 Refer to latest copy of the Noist product information details for framing details, sufferent tables, web hole chart, bridging details, multi-ply fastening details and handling/erection details
 Damaged Noists must not be used

Curreyed noists must not be used Design assumes top flange to be laterally restrained by attached shealthing or as specified in engineering notes.

Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing lengther 3.5 inches
 For flat roofs provide proper drainage to prevent populario.

Manufacturer Info

Nascor by Kott

Kott Lumber Company 14 Anderson Blvd, Ontario K2H7V1 905-642-4400





BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS.

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the linead application, and to verify the dimensions and loads.

### Lumber

Dry service conditions, unless noted otherwise
 Upist not to be treated with fire retardant or corrosive

chemicals

### Handling & Installation

- Handling & Installation

  1. Jioist flanges must not be cut or drilled

  2. Refer to latest copy of the Jioist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-by festening details and handlingferection details

  3. Damaged Jobists must not be used

  4. Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
   Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
   For flat roofs provide proper drainage to prevent pondling

Manufacturer Info

Nascor by Kott

Kott Lumber Company 14 Anderson Blvd, Ontario Canada K2H7V1 905-642-4400







Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the Intended application, and to verify the dimensions and loads. Lumber

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corr

chemicals

Handling & Installation

and ling & Installation

Jost langes must not be cut or drilled

Refer to Islest copy of the Lioist product information
details for framing details, stiffered tables, web hole
chart, bridging details, multi-phy fastening details and
handling/erection details

Damaged Loists must not be used

Design assumes top flange to be laterally restrained
by attached sheathing or as specified in engineering
notes.

Provide lateral support at bearing points to avoid lateral displacement and rotation

lateral displacement and rotation

6. Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches

7. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

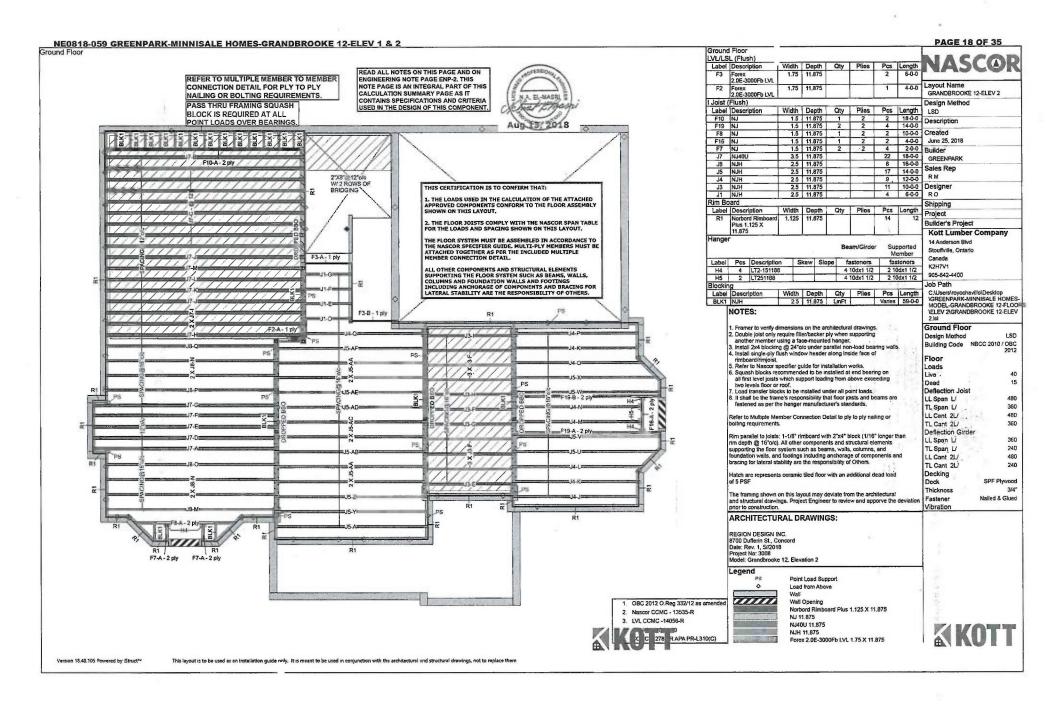
Nascor by Kott

Kott Lumber Company 14 Anderson Blvd, Ontario K2H7V1 905-642-4400









GREENPARK

Page 1 of 1



isDesign<sup>™</sup>

Client:

Project:

Address:

Date:

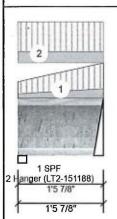
Designer: RO

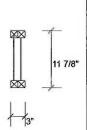
Job Name: GRANDBROOKE 12-ELEV 2

Project #:

2-Ply - PASSED NJ 11.875"

Level: Ground Floor





Member Informati	on				Unfacto	red Reac	tions U	INPATTERNI	ED Ib	(Uplift)	
Type: Gire Plies: 2 Moisture Condition: Dry Deflection LL: 360 Deflection TL: 240 Importance: Nor General Load Floor Live: 40	mal	Application Design Mel Building Co Load Shari Deck: Vibration:	thod: LS ode: N ng: N	loor (Residential) SD BCC 2010 / OBC 201: o ot Checked ot Checked		Live 74 84 s and Fac		Dead 28 31	Sno	w 0 0	Wind 0 0
Dead: 15 Analysis Results	PSF				Bearing 1 - SPF 2 - Hanger	_	Cap. 5% 6%		Total 146 165	-	Ld. Comb. 1.25D+1.5L 1.25D+1.5L
Analysis Actual Moment 44 ft-lb Unbraced 44 ft-lb Shear 140 lb Perm Defl in. 0.000 (L LL Defl inch 0.000 (L TL Defl inch 0.000 (L	999) 0	9020 ft-lb 6 8539 ft-lb	0.005 (1%) 0.041 (4%) 0.000 (0%) 0.000 (0%)	Comb. Case 1.25D+1.5L L 1.25D+1.5L L 1.25D+1.5L L	REFER TOONNED NAILING PASS TO BLOCK I POINT LO	TION DET OR BOLT IRU FRAM S REQUIR OADS OVE	AIL FOR ING REC ING SQU ED AT A ER BEAF	ILL RINGS. PAGE AND O		189	EL-MASRI E
Design Notes  1 Fill all hanger nailing 2 Girders are designed 3 Multiple plies must be 4 Top loads must be su 5 Top flange braced at 6 Bottom flange braced	to be supported on the fastened together as pported equally by all pearings.	s per manufactur			NOTE PA CALCUL CONTAI	AGE IS AN ATION SUI NS SPECIF	INTEGR MMARY ICATION	E ENP-2. THIS AL PART OF T PAGE AS IT IS AND CRITEI HIS COMPONE	RIA	Aug	3; 2018

### Notes

1

2

Calculated Shuchared Designs is responsible only of the shuckural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the inlended application, and to verify the dimensions and loads. Lumber

Tie-In

Tie-In

Dry service conditions, unless noted otherwise
 Uoist not to be treated with fire retardant or corrosive

Handliing & Installation

1. Uoist flanges must not be cut or drilled

2. Refer to latest copy of the Libest product information details for framing details, stifferer tables, web hole chart, bridging details, multi-ply festering details and handling/erection details

3. Damaged Uoist must not be used

4. Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

0-0-0 to 1-5-14 (Span)1-3-12 Top

0-0-0 to 1-5-14 (Span)3-3-0 Top

to 2-9-10

15 PSF

15 PSF

40 PSF

40 PSF

Manufacturer Info

Nascor by Kott

0 PSF

0 PSF

Kott Lumber Company 14 Anderson Blvd, Ontario K2H7V1 905-642-4400



0 PSF

0 PSF



### NE0818-059 GREENPARK-MINNISALE HOMES-GRANDBROOKE 12-ELEV 1 & 2

isDesign™

Client:

**GREENPARK** 

8/10/2018

Page 1 of 1

**PAGE 21 OF 35** 

Project:

Address:

Designer: RO

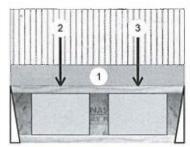
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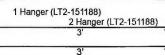
Project #:

11.875"

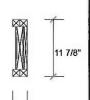
2-Ply - PASSED

Level: Ground Floor





15 PSF



Wind

0

0

Ld. Comb.

1.25D+1.5L

1.25D+1.5L

Member Inform	nation		
Type:	Girder	Application:	Floor (Residential)
Plies:	2	Design Method:	LSD
Moisture Condition	: Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		

Brg Live 311 116 0 319 119 0 2

Unfactored Reactions UNPATTERNED Ib (Uplift)

Dead

Cap. React D/L lb

145 / 466

149 / 478

Snow

Total Ld. Case

611 L

628 L

### **Analysis Results Analysis** Actual Location Allowed Capacity Comb. Case Moment 445 ft-lb 1'3 1/4" 9020 ft-lb 0.049 (5%) 1.25D+1.5L L 445 ft-lb 1'3 1/4" 5749 ft-lb Unbraced 0.077 (8%) 1.25D+1.5L L 620 lb 2'10 3/4" 3400 lb 0.182 (18%) 1.25D+1.5L L Shear Perm Defl in. 0.001 1'5 3/16" 0.093 (L/360) 0.010 (1%) D Uniform (L/34461)LL Defl inch 0.003 1'5 3/16" 0.093 (L/360) 0.030 (3%) L L (L/12881) TL Defl inch 0.004 (L/9376) 1'5 3/16" 0.140 (L/240) 0.030 (3%) D+L

Hanger REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

23%

23%

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS.

Bearings and Factored Reactions

Bearing Length

Hanger

2 -

2.000"

2.000"

READ ALL NOTES ON THIS PAGE AND ON ENGINEERING NOTE PAGE ENP-2. THIS NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT **CONTAINS SPECIFICATIONS AND CRITERIA** USED IN THE DESIGN OF THIS COMPONENT.



### **Design Notes**

Dead:

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top flange braced at bearings.
- 6 Bottom flange braced at bearings.

145-1-0
ow Wind Comments
SF 0 PSF
0 lb
0 lb 0 lb J4
0

### Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer end/or the contractor to ensure the component suitability of the intended

### Lumber

### Handling & Installation

Handling & Installation

1. Joist flanges must not be cut or drilled

2. Refer to latest copy of the Lioist product information details for framing details, eitherer tables, web hole chart, bridging details, multi-ply tastening details and handling/erection details

3. Damaged bolists must not be used

4. Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

Provide lateral support at bearing points to avoid tateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing singlibr=3.5 inches
 Teo flat roofs provide proper dislinage to prevent ponding

Manufacturer Info

Nascor by Kott

Kott Lumber Company 14 Anderson Blvd, Ontario Canada K2H7V1







GREENPARK Client:

Project: Address:

8/10/2018

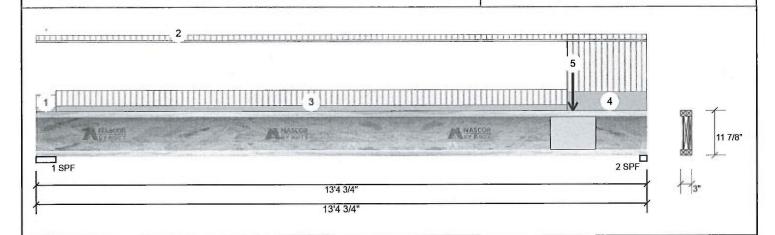
RO

Job Name: GRANDBROOKE 12-ELEV 2

Designer: Project #:

11.875" 2-Ply - PASSED NJ

Level: Ground Floor



Type:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	1	Wind
Plies:	2	Design Method:	LSD	1	222	83	(	)	0
Moisture Conditio	n: Dry	Building Code:	NBCC 2010 / OBC 2012	2	523	196	(	)	0
Deflection LL:	360	Load Sharing:	No						
Deflection TL:	240	Deck:	Not Checked						
Importance:	Normal	Vibration:	Not Checked						
General Load									
Floor Live:	40 PSF			Bearings	and Fac	tored Reactions			
Dead:	15 PSF			Bearing	Length	Cap. React D/L lb	Total	Ld. Case	Ld. Comb.
				1 - SPF	5.250"	13% 104 / 333	437	L	1.25D+1.5L
				2-SPF	1.875"	39% 245 / 785	1029	L	1.25D+1.5L

**Analysis Results** 

Member Information

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1685 ft-lb	8'4 5/8"	9020 ft-lb	0.187 (19%)	1.25D+1.5L	L
Unbraced	1685 ft-lb	8'4 5/8"	1686 ft-lb	0.999 (100%)	1.25D+1.5L	L
Shear	1016 lb	13'3 5/8"	3400 lb	0.299 (30%)	1.25D+1.5L	L
Perm Defl in.	0.022 (L/6937)	7'3 1/16"	0.431 (1/360)	0.050 (5%)	D	Uniform
LL Defl inch	0.060 (L/2597)	7'3 1/16"	0.431 (1/360)	0.140 (14%)	L	L
TL Defl inch	0.082 (L/1889)	7'3 1/16"	0.646 (L/240)	0.130 (13%)	D+L	L

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

**Unfactored Reactions UNPATTERNED Ib (Uplift)** 

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS.

READ ALL NOTES ON THIS PAGE AND ON **ENGINEERING NOTE PAGE ENP-2. THIS** NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT **CONTAINS SPECIFICATIONS AND CRITERIA** USED IN THE DESIGN OF THIS COMPONENT.



### **Design Notes**

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 5'6" o.c.

	5 Bottom llange	braced at bearings								
	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
	1	Tie-In	0-0-0 to 0-5-4	(Span)0-9-6	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	2	Tie-In	0-0-0 to 13-4-12	(Span)0-4-2	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	3	Tie-In	0-5-4 to 11-7-14	(Span) 0-11-14	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	4	Tie-In	11-7-14 to 13-4-12	(Span)3-3-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
ŀ	5	Point	11-9-6		Far Face	116 lb	311 lb	0 lb	0 lb	F16

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

### Lumber

Dry service conditions, unless noted otherwise
 Uoist not to be treated with fire retardant or corrosive

- Handling & Installation

  1. IJoist flanges must not be cut or drilled
  2. Refer to letest copy of the LiJoist product information details for framing details, suffarer to letes, web hole chart, bridsing details, multi-ply isstending details and handling/rection details.
  3. Damaged LiJoist must not be used
  4. Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering nodes.

Manufacturer Info

Kott Lumber Company 14 Anderson Blvd, Ontario Canada K2H7V1 905-642-4400





Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
Tie-In	0-0-0 to 11-7-14	(Span)0-11-2	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
Tie-In	0-5-4 to 13-4-12	(Span)0-4-14	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
Tie-In	11-7-14 to 13-4-12	(Span)3-3-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
Point	11-9-6		Near Face	119 lb	319 lb	0 lb	0 lb	F16
	Tie-In Tie-In Tie-In	Tie-In 0-0-0 to 11-7-14 Tie-In 0-5-4 to 13-4-12 Tie-In 11-7-14 to 13-4-12	Tie-In 0-0-0 to 11-7-14 (Span)0-11-2 Tie-In 0-5-4 to 13-4-12 (Span)0-4-14 Tie-In 11-7-14 to 13-4-12 (Span)3-3-0	Tie-In 0-0-0 to 11-7-14 (Span)0-11-2 Top Tie-In 0-5-4 to 13-4-12 (Span)0-4-14 Top Tie-In 11-7-14 to 13-4-12 (Span)3-3-0 Top	Tie-In         0-0-0 to 11-7-14         (Span)0-11-2         Top         15 PSF           Tie-In         0-5-4 to 13-4-12         (Span)0-4-14         Top         15 PSF           Tie-In         11-7-14 to 13-4-12         (Span)3-3-0         Top         15 PSF	Tie-In         0-0-0 to 11-7-14         (Span)0-11-2         Top         15 PSF         40 PSF           Tie-In         0-5-4 to 13-4-12         (Span)0-4-14         Top         15 PSF         40 PSF           Tie-In         11-7-14 to 13-4-12         (Span)3-3-0         Top         15 PSF         40 PSF	Tie-In         0-0-0 to 11-7-14         (Span)0-11-2         Top         15 PSF         40 PSF         0 PSF           Tie-In         0-5-4 to 13-4-12         (Span)0-4-14         Top         15 PSF         40 PSF         0 PSF           Tie-In         11-7-14 to 13-4-12         (Span)3-3-0         Top         15 PSF         40 PSF         0 PSF	Tie-In         0-0-0 to 11-7-14         (Span)0-11-2         Top         15 PSF         40 PSF         0 PSF         0 PSF           Tie-In         0-5-4 to 13-4-12         (Span)0-4-14         Top         15 PSF         40 PSF         0 PSF         0 PSF           Tie-In         11-7-14 to 13-4-12         (Span)3-3-0         Top         15 PSF         40 PSF         0 PSF         0 PSF

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the daspn criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads. Lumber

Dry service conditions, unless noted otherwise
 Uplist not to be treated with fire retardant or corrosive

### chemicals

- andling & Installation

  Joist langes must not be cut or drilled

  Refer to latest copy of the Lloist product information details for framing details, stiffener tables, web hote chart. bridging details, multi-phy fastening details and handling/erection details

  Damaged Lloist must not be used

  Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeness for point load as shown Minimum point load bearing lengther 3.5 inches
 Por flat roofs provide proper drainage to prevent ponding

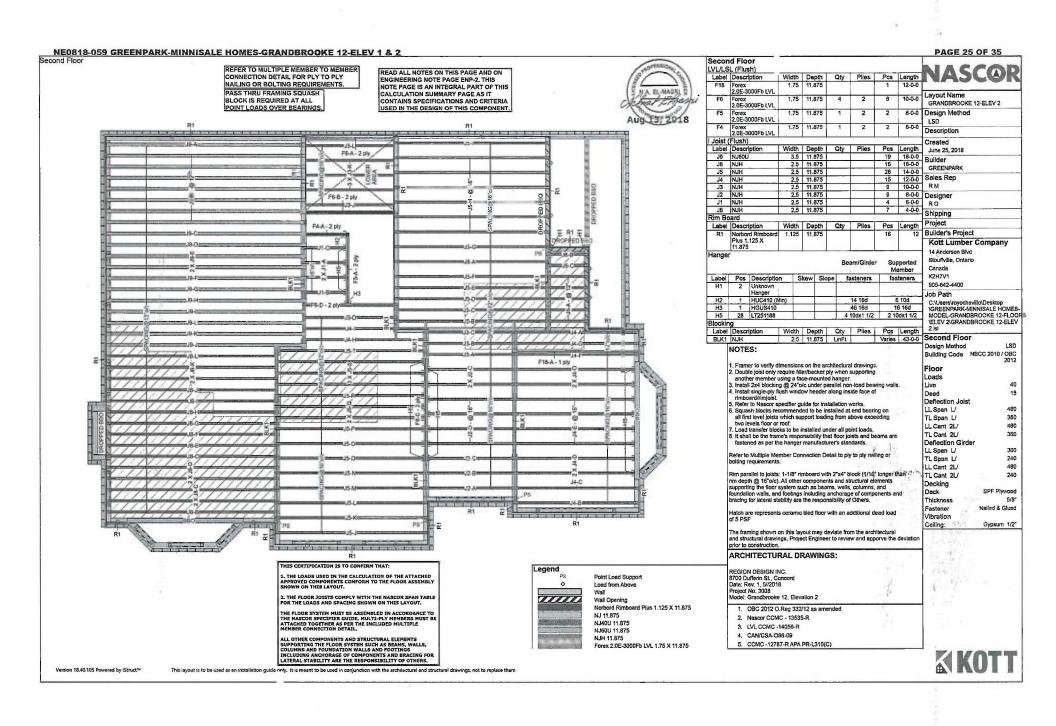
Manufacturer Info

Nascor by Kott

Kott Lumber Company 14 Anderson Blvd, Ontario 905-642-4400







isDesign™

Client:

Address:

Project:

Date:

Designer: RO

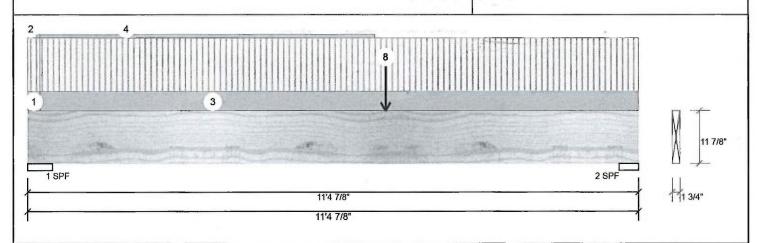
Job Name: GRANDBROOKE 12-ELEV 1

Project #:

Forex 2.0E-3000Fb LVL F18-A

1.750" X 11.875" - PASSED

Level: Second Floor



Member Info	rmation		Unfactored Reactions UNPATTERNED Ib (Uplift)							
Type:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snov	v	Wind
Plies:	1	Design Method:	LSD	1	98		167	148	В	0
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	103		206	213	2	0
Deflection LL:	360	Load Sharing:	No	1 -						
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal	Vibration:	Not Checked	1						
General Load										
Floor Live:	40 PSF			Bearings	and Fact	tored F	leactions			
Dead:	15 PSF			Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
				1 - SPF	5.500"	8%	208 / 272	480	L	1.25D+1.5S +0.5L
Analysis Resu		ation Allowed Canac	ity Comb Case	2-SPF	4.375"	13%	258 / 369	626	L	1.25D+1.5S +0.5L

Analysis Actual Location Allowed Capacity Comb. 2542 ft-lb 6'8 1/4" 17130 ft-lb 0.148 (15%) 1.25D+1.5S L Moment +0.5L Unbraced 2542 ft-lb 6'8 1/4" 4199 ft-lb 0.605 (61%) 1.25D+1.5S L +0.5L 0.104 (10%) 1.25D+1.5S L 10'1 3/8" 5798 lb Shear 600 lb +0.5L Perm Defl in. 0.032 (L/3978) Uniform 6'2 1/16" 0.357 (L/360) 0.090 (9%) D 6'2 7/8" 0.357 (L/360) 0.120 (12%) S+0.5L LL Defl inch 0.043 (L/3012) L TL Defl inch 0.075 (L/1714) 6'2 1/2" 0.535 (L/240) 0.140 (14%) D+S+0.5L

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.

READ ALL NOTES ON THIS PAGE AND ON **ENGINEERING NOTE PAGE ENP-2. THIS** NOTE PAGE IS AN INTEGRAL PART OF THIS **CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA** USED IN THE DESIGN OF THIS COMPONENT.



Page 1 of 2

### **Design Notes**

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Top braced at bearings.
- 3 Bottom braced at bearings

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 0-2-12	(Span)0-8-13	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Part, Uniform	0-0-0 to 0-2-12		Тор	1 PLF	0 PLF	0 PLF	0 PLF	
3	Tie-In	0-2-12 to 11-4-14	(Span)0-8-13	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
4	Part. Uniform	0-2-12 to 6-5-13		Тор	1 PLF	0 PLF	0 PLF	0 PLF	
5	Point	6-8-4		Тор	204 lb	33 lb	329 lb	0 lb	F16 F16
6	Point	6-8-4		Тор	7 lb	0 lb	17 lb	0 lb	
7	Point	6-8-4		Тор	6 lb	0 lb	14 lb	0 lb	

Continued on page 2...

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the Intended application, and to verify the dimensions and loads. Lumber

chemicals

Handling & Installation

- LVL beams must not be cut or drilled
   Refer to manufacturer's product information regarding installation requirements, multi-pty fastening details, beam strength values, and code
- fastening waters, approvals approvals Damaged Beams must not be used Design assumes top edge is laterally restrelined Provide lateral support at bearing points to lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Forex APA: PR-L318

Manufacturer Info

Kott Lumber Company 14 Anderson Blvd, Ontario Canada K2H7V1 905-642-4400



NE0818-059 GREENPARK-MINNISALE HOMES-GRANDBROOKE 12-ELEV 1 & 2 **PAGE 27 OF 35** GREENPARK Client: Date: Page 2 of 2 Designer: RO Project: isDesign<sup>™</sup> Address: Job Name: GRANDBROOKE 12-ELEV 1 Project #: 1.750" X 11.875" - PASSED Level: Second Floor Forex 2.0E-3000Fb LVL F18.-A 2 4 3 11 7/8" 1 SPF 2 SPF 1 3/4" 11'4 7/8' 11'4 7/8" .Continued from page 1 ID Load Type Location Trib Width Side Dead Live Snow Wind Comments 8 Point 6-8-4 Top 32 lb 0 lb 0 lb 0 lb Wall Self Weight Self Weight 5 PLF

> REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS.

### Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads. Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire relardant or corrosive

### chemicals

- Handling & Installation

  1. LVL beam must not be cut or drilled

  2. Refer to manufacturer's product Information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

  3. Damsged Beams must not be used

  4. Design assumes top edge is laterally restrained

  5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent

Manufacturer Info

Forex APA: PR-L318

Kott Lumber Company 14 Anderson Blvd, Onlario Canada K2H7V1 905-642-4400



KOTT NASCOI



Client:

Address:

**GREENPARK** Project:

Date:

8/10/2018

Page 1 of 1

Designer: RO

Job Name: GRANDBROOKE 12-ELEV 1

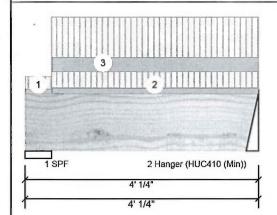
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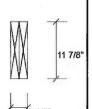
Forex 2.0E-3000Fb LVL

1.750" X 11.875"

2-Ply - PASSED

Level: Second Floor





Member Info	rmation		
Type:	Girder	Application:	Floor (Residential)
Plies:	2	Design Method:	LSD
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		

Brg	Live	Dead	Snow	Wind
1	149	76	0	0
2 2	158	76	0	0

Cap. React D/L lb

94 / 224

95 / 237

Analysis Res	sults					
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	267 ft-lb	2'1 5/8"	34261 ft-lb	0.008 (1%)	1.25D+1.5L	L
Unbraced	267 ft-lb	2'1 5/8"	34261 ft-lb	0.008 (1%)	1.25D+1.5L	L
Shear	132 lb	1'4 5/8"	11596 lb	0.011 (1%)	1.25D+1.5L	L
Perm Defl in.	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
LL Defl inch	0.001 (L/65763)	2'1 11/16"	0.116 (L/360)	0.010 (1%)	L	L
TL Defl inch	0.001 (L/44356)	2'1 11/16"	0.174 (L/240)	0.010 (1%)	D+L	L

Hanger REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

3%

5%

Bearings and Factored Reactions

Bearing Length

1 - SPF 5.500"

2 -

2.500"

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS.

READ ALL NOTES ON THIS PAGE AND ON **ENGINEERING NOTE PAGE ENP-2. THIS** NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



Ld. Comb.

1.25D+1.5L

1.25D+1.5L

Total Ld. Case

319 L

333 L

### **Design Notes**

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on full section width.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 0-5-8	(Span)0-11-6	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-5-8 to 4-0-4	(Span)1-2-6	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Part. Uniform	0-5-8 to 4-0-4		Тор	22 PLF	60 PLF	0 PLF	0 PLF	
	Self Weight				10 PLF				

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended

### Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

Handling & Installation

1. UVI, beams must not be cut or drilled

2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

3. Damagade Beams must not be used

4. Design assumes top edge is laterally restrained

5. Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318

Kott Lumber Company 14 Anderson Blvd, Ontario Canada K2H7V1 905-642-4400





Page 1 of 2



Address:

**GREENPARK** Project:

8/10/2018

Designer: RO

Project #:

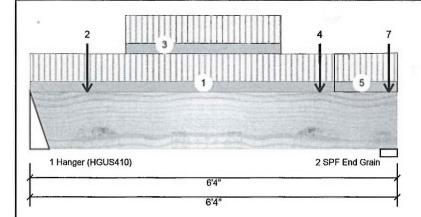
Forex 2.0E-3000Fb LVL

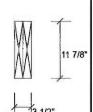
1.750" X 11.875"

2-Ply - PASSED

Level: Second Floor

Job Name: GRANDBROOKE 12-ELEV 1





er	Application: Design Method: Building Code:	Floor (Residential) LSD NBCC 2010 / OBC 2012
	Building Code:	NBCC 2010 / OBC 2012
		14000 20101 000 2012
	Load Sharing:	No
	Deck:	Not Checked
mal	Vibration:	Not Checked
PSF		
PSF		
)	SF	Deck: Vibration:

<b>Unfactored</b>	Reactions	UNPAT	TERNED	lb i	(Uplift)

Live	Dead	Snow	Wind
442	196	0	0
582	264	0	0
	442	442 196	442 196 0

### **Analysis Results**

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case	
Moment	1280 ft-lb	3'2 1/8"	34261 ft-lb	0.037 (4%)	1.25D+1.5L	L	
Unbraced	1280 ft-lb	3'2 1/8"	32678 ft-lb	0.039 (4%)	1.25D+1.5L	L	
Shear	709 lb	1'3 1/8"	11596 lb	0.061 (6%)	1.25D+1.5L	L	
Perm Defl in.	0.002 (L/28206)	3'2 1/4"	0.194 (᠘360)	0.010 (1%)	D	Uniform	
LL Defl inch	0.006 (L/12392)	3'2 1/4"	0.194 (L/360)	0.030 (3%)	L	L	
TL Defl inch	0.008 (L/8610)	3'2 1/4"	0.292 (L/240)	0.030 (3%)	D+L	L	

### **Bearings and Factored Reactions**

bearing.	and ruc	torca	teactions			
Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - Hanger	4.000"	9%	245 / 663	908	L	1.25D+1.5L
2 - SPF End Grain	3.500"	13%	330 / 873	1203	L	1.25D+1.5L

### **Design Notes**

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.
- 2 Fill all hanger nailing holes.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Multiple plies must be fastened together as per manufacturer's details.
- 5 Top loads must be supported equally by all plies.
- 6 Top braced at bearings.
- 7 Bottom braced at bearings.
- 8 Lateral slenderness ratio based on full section width

READ ALL NOTES ON THIS PAGE AND ON **ENGINEERING NOTE PAGE ENP-2. THIS** NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS.



O Laterai	Sichachicas land pasca	on full section with.							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 5-2-14	(Span)3-8-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	0-11-14		Far Face	35 lb	92 lb	0 lb	0 lb	J1
3	Part. Uniform	1-7-14 to 4-3-14		Far Face	28 PLF	75 PLF	0 PLF	0 PLF	

Continued on page 2...

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the Intended Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

  1. LVL beams must not be cut or drilled

  2. Refer to manufacturer's product information regarding installation requirements. multi-ply fastening details, beam strength values, and code approvals

  3. Damaged Beams must not be used

  4. Design assumes top edge is laterally restrained

  5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent

Manufacturer Info APA: PR-L318

Kott Lumber Company 14 Anderson Blvd, Ontario K2H7V1 905-642-4400

Page 2 of 2

isDesign™

Project:

**GREENPARK** Address:

8/10/2018

Designer: RO

Job Name: GRANDBROOKE 12-ELEV 1

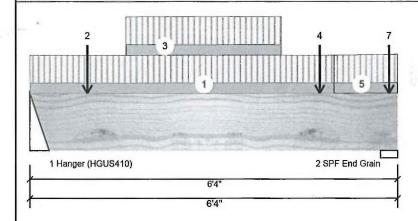
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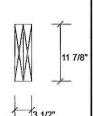
Forex 2.0E-3000Fb LVL

1.750" X 11.875"

2-Ply - PASSED

Level: Second Floor





Continued	from page 1								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
4	Point	4-11-14		Far Face	35 lb	94 lb	0 lb	0 lb	J1
5	Tie-In	5-2-14 to 6-4-0	(Span)3-8-8 to 3-9-10	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
6	Point	6-2-4		Тор	3 lb	9 lb	0 lb	0 lb	
7	Point	6-2-4		Far Face	76 lb	158 lb	0 lb	0 lb	F4
	Self Weight				10 PLF				

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer endfor the contractor to ensure the component suitability of the Intended application, and to verify the dimensions and loads.

### Lumber

Handling & Installation

Handling & Installation

1. LVL beams must not be cut or drilled

2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

3. Damaged Beams must not be used

4. Design assumes top edge is laterally restrained

5. Provide lateral support at bearing points to avoid lateral displacement and rotation

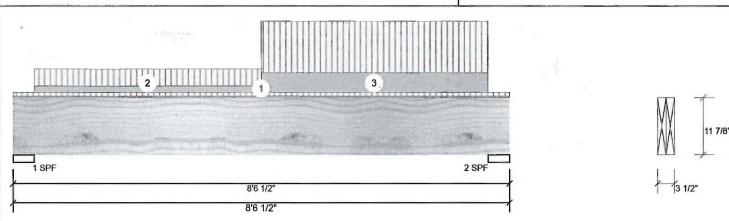
This design is valid until 7/10/2021

Manufacturer Info

Forex APA: PR-L318

Kott Lumber Company 14 Anderson Blvd, Ontario K2H7V1 905-642-4400





Туре:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind
Plies:	2	Design Method:	LSD	1	411	194	0	0
Moisture Condition:	: Dry	Building Code:	NBCC 2010 / OBC 2012	2	642	282	0	0
Deflection LL:	360	Load Sharing:	No	_				
Deflection TL:	240	Deck:	Not Checked					
Importance:	Normal	Vibration:	Not Checked					
General Load								
Floor Live:	40 PSF			Bearings	and Fact	ored Reactions		
Dead:	15 PSF			Bearing I	Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
				1 - SPF 4	4.375"	9% 242 / 617	860 L	1.25D+1.5L
				2-SPF 4	4 375"	14% 353 / 963	1316 L	1.25D+1.5L

**Analysis Results** Analysis Location Allowed Capacity Comb. Case Moment 2235 ft-lb 4'10 7/16" 34261 ft-lb 0.065 (7%) 1.25D+1.5L L Unbraced 2235 ft-lb 4'10 7/16" 31329 ft-lb 0.071 (7%) 1.25D+1.5L L 0.081 (8%) 1.25D+1.5L L Shear 937 lb 7'3" 11596 lb Perm Defl in. 0.007 4'5 7/8" 0.265 (L/360) 0.030 (3%) D Uniform (L/14057) 4'6 1/4" 0.265 (L/360) 0.060 (6%) L LL Defl inch 0.015 (L/6266) L 4'6 1/8" 0.397 (L/240) 0.060 (6%) D+L TL Defl inch 0.022 (L/4334) L

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS

**Unfactored Reactions UNPATTERNED lb (Uplift)** 

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS.

READ ALL NOTES ON THIS PAGE AND ON **ENGINEERING NOTE PAGE ENP-2. THIS** NOTE PAGE IS AN INTEGRAL PART OF THIS **CALCULATION SUMMARY PAGE AS IT** CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



1 Girders are designed to be supported on the bottom edge only. 2 Multiple plies must be fastened together as per manufacturer's details.

3 Top loads must be supported equally by all plies.

4 Top braced at bearings.

**Design Notes** 

Member Information

5 Bottom braced at bearings

6 Lateral slenderness ratio based on full section width.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 8-6-8	(Span)0-8-2	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Part. Uniform	0-4-6 to 4-3-4		Тор	22 PLF	60 PLF	0 PLF	0 PLF	
3	Part. Uniform	4-3-4 to 8-2-2		Тор	68 PLF	180 PLF	0 PLF	0 PLF	
	Self Weight				10 PLF				

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the Intended application, and to verify the dimensions and loads. Lumber

chemicals

Handling & Installation

1. LVL beams must not be cut or drilled
 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code

tatactum y supports approvals

3. Damaged Beams must not be used

4. Design assumes top edge is laterally restrained

5. Provide lateral support at bearing points to avoid lateral displacement and rotation

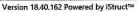
6. For flat roofs provide proper drainage to prevent

Manufacturer Info

APA: PR-I 318

Kott Lumber Company 14 Anderson Blvd, Onterio Canada K2H7V1 905-642-4400









Address:

GREENPARK Project:

8/10/2018

Designer: RO

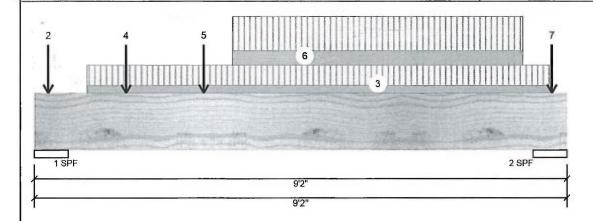
Job Name: GRANDBROOKE 12-ELEV 1

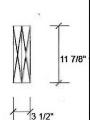
Project #:

Forex 2.0E-3000Fb LVL F6-C

1.750" X 11.875" 2-Ply - PASSED

Level: Second Floor





ñ	V	en	nb	er	In	fo	rm	at	io	n
	٠.	-		~.				-		

Type:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal
General Load	
Floor Live:	40 PSF

15 PSF

Floor (Residential)

LSD NBCC 2010 / OBC 2012 No

Not Checked Not Checked

### Unfactored Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	Wind
1	1914	784	0	0
2	1861	791	0	0

### **Bearings and Factored Reactions**

Bearing Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1-SPF 7.000"	26%	980 / 2871	3851	L	1.25D+1.5L
2-SPF 7.000"	25%	989 / 2792	3781	L	1.25D+1.5L

Analysis Results

Dead:

	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
	Moment	7003 ft-lb	4'7 1/8"	34261 ft-lb	0.204 (20%)	1.25D+1.5L	L
	Unbraced	7003 ft-lb	4'7 1/8"	31189 ft-lb	0.225 (22%)	1.25D+1.5L	L
١	Shear	3621 lb	1'6 1/8"	11596 lb	0.312 (31%)	1.25D+1.5L	L
	Perm Defl in.	0.022 (L/4500)	4'7 3/16"	0.271 (L/360)	0.080 (8%)	D	Uniform
	LL Defl inch	0.052 (L/1889)	4'7 1/16"	0.271 (L/360)	0.190 (19%)	L	L
	TL Defl inch	0.073 (L/1330)	4'7 1/16"	0.406 (L/240)	0.180 (18%)	D+L	L
г							

Application:

Design Method: **Building Code:** 

Load Sharing:

Deck: Vibration:

> REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH BLOCK IS REQUIRED AT ALL POINT LOADS OVER BEARINGS

READ ALL NOTES ON THIS PAGE AND ON **ENGINEERING NOTE PAGE ENP-2. THIS** NOTE PAGE IS AN INTEGRAL PART OF THIS **CALCULATION SUMMARY PAGE AS IT** CONTAINS SPECIFICATIONS AND CRITERIA



### **Design Notes**

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top braced at bearings.
- 4 Rottom braced at bearings.
- 5 Lateral slenderness ratio based on full section width

USED IN THE DESIGN OF THIS COMPONENT.



### Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, mutti-ply fastening details, beam strength values, and code
- approvals Damaged Beams must not be used
- Damaged beams must not be deed Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prever poorfing

This design is valid until 7/10/2021

APA: PR-L318

Manufacturer Info

Kott Lumber Company 14 Anderson Blvd, Ontario K2H7V1 905-642-4400





Client: Project:

Address:

GREENPARK

Date:

Designer: RO

Job Name: GRANDBROOKE 12-ELEV 1

**Unfactored Reactions UNPATTERNED Ib (Uplift)** 

Dead

201

Snow

0

0

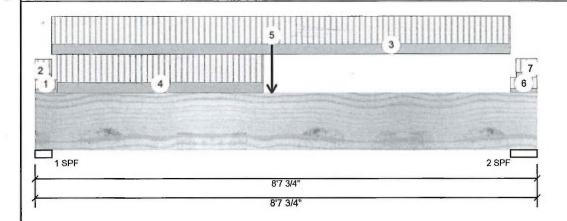
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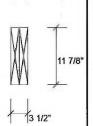
F6-D Forex 2.0E-3000Fb LVL 1.750" X 11.875"

2-Ply - PASSED

Brg

Level: Second Floor





Wind

0

0

Member Information					
Туре:	Girder				
Plies:	2				
Moisture Condition:	Dry				
Deflection LL:	360				
Deflection TL:	240				
Importance:	Normal				
General Load					
Floor Live:	40 PSF				
Dead:	15 PSF				

Floor (Residential) Application: Design Method: **Building Code:** NBCC 2010 / OBC 2012 Load Sharing: No

Not Checked

Not Checked

328 179 2

Live

386

**Bearings and Factored Reactions** Ld. Comb. Cap. React D/L lb Bearing Length Total Ld. Case 1 - SPF 3.500" 11% 251 / 578 829 L 1.25D+1.5L 2 - SPF 5.500" 6% 224 / 492 1.25D+1.5L 716 L

**Analysis Results** 

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2425 ft-lb	4'1"	34261 ft-lb	0.071 (7%)	1.25D+1.5L	L
Unbraced	2425 ft-lb	4'1"	31268 ft-lb	0.078 (8%)	1.25D+1.5L	L
Shear	723 lb	1'2 5/8"	11596 lb	0.062 (6%)	1.25D+1.5L	L
Perm Deft in.	0.007 (L/13103)	4'1 1/16"	0.267 (∐360)	0.030 (3%)	D	Uniform
LL Defl inch	0.015 (L/6579)	4'1 1/16"	0.267 (L/360)	0.050 (5%)	L	L
TL Defl inch	0.022 (L/4380)	4'1 1/16"	0.401 (L/240)	0.050 (5%)	D+L	L

Deck:

Vibration:

REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.

READ ALL NOTES ON THIS PAGE AND ON **ENGINEERING NOTE PAGE ENP-2. THIS** NOTE PAGE IS AN INTEGRAL PART OF THIS CALCULATION SUMMARY PAGE AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



**Design Notes** 

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on full section width.

o Lateral deligeness ratio based on fall dedict matti.									
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 0-4-8	(Span)0-4-10	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 0-3-8	(Span)0-7-6	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Tie-In	0-3-8 to 8-2-4	(Span)1-1-6	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
4	Tie-In	0-4-8 to 3-11-4	(Span)1-1-10	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
5	Point	4-1-0		Far Face	196 lb	442 lb	0 lb	0 lb	F5
6	Tie-In	8-2-4 to 8-7-12	(Span)0-5-6	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

Continued on page 2...

Notes

Calculated Shuctured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

LVL beams must not be cut or drilled
Refer to manufacturer's product information
regarding installation requirements, multi-ply
fastening details, beam strength values, and code
approvals
Damaged Beams must not be used

Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation

This design is valid until 7/10/2021

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info Forex

APA: PR-L318

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REFER TO MULTIPLE MEMBER TO MEMBER CONNECTION DETAIL FOR PLY TO PLY NAILING OR BOLTING REQUIREMENTS.

PASS THRU FRAMING SQUASH **BLOCK IS REQUIRED AT ALL** POINT LOADS OVER BEARINGS.

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