

Engineering Note Page (ENP-2)

REVISION 2009-10-09

Please read all notes prior to installation of the component**DESIGN INFORMATION**

This building component is certified as an individual component for the loads and conditions shown on the calculation and drawing page.

The responsibility of the undersigned engineer is only limited to the calculation of this building component for the loads and conditions shown on this drawing.

The responsibility of the undersigned is limited to the verification of the structural capacity of the NASCOR floor joists and LVL beams based on placement as shown on the layout. The loads applied are limited to the gravity effects of the specified loads. The structural integrity of the building and the effect of wind, uplift, seismic, lateral or other forces, calculation of adequate support and anchorage of components, as well as the dimensions and design loads used to calculate components are the responsibility of the overall building designer.

Floor joists and OSB rim board are designed to carry uniformly distributed loads only. Point loads should be transferred through the floor cavity with squash blocks. Structural elements such as walls, posts, connectors, and squash blocks are the responsibility of the overall building designer.

The undersigned engineer disclaims any responsibility for damages as a result of being furnished faulty or incorrect information, specifications and/or designs.

Installation of NASCOR joists is to be carried out in accordance with the current edition of the manufacturer's approved literature available at <http://www.nascor.ca>.

CODE

This building component is designed in accordance with the National Building Code of Canada, the Ontario Building Code, CCMC and Canadian Standards Association guidelines.

COMPONENT

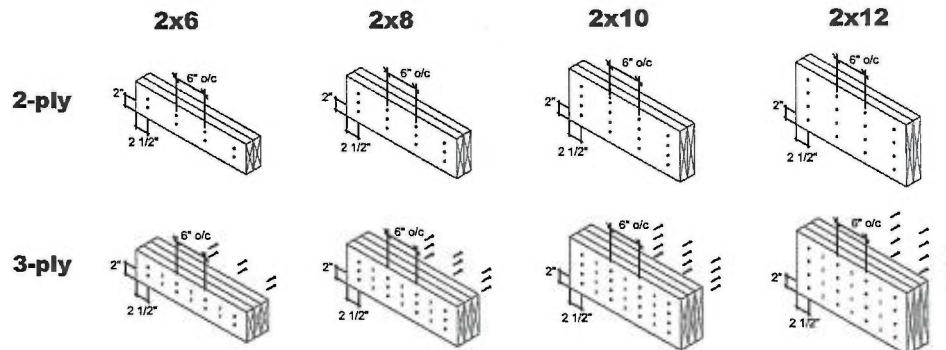
1. The building component used in construction must be the same as indicated on the drawings.
2. The building component must be installed and assembled as per specification shown on the drawing and in accordance with the manufacturer's assembly and installation.
3. Members consisting of multiple plies must be connected as per the document "Multi-ply Connection Details".
4. Pass-thru squash block framing is required at all point loads over bearings.

HANDLING AND INSTALLATION

Do not drill any hole, cut or notch a certified building component without a written pre-authorization.

MULTIPLE MEMBER CONNECTIONS

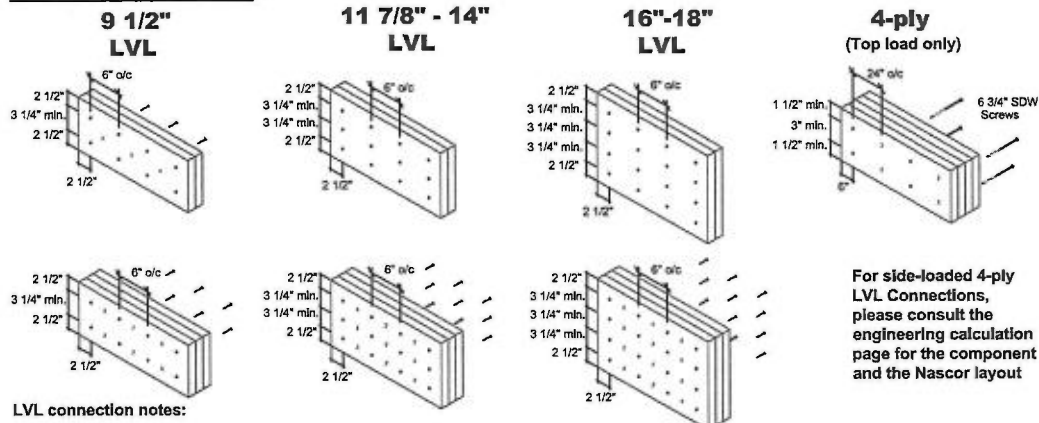
Conventional Connections (for uniform distributed loads)



Conventional connection notes:

- Nails to be 3" 10d spiral wire nails.
- Nails to be located a minimum of 2" from the top and bottom of the member. Start all nails a minimum of 2 1/2" in from ends.
- Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail driven from the opposite side.

LVL Connections (for uniform distributed loads)

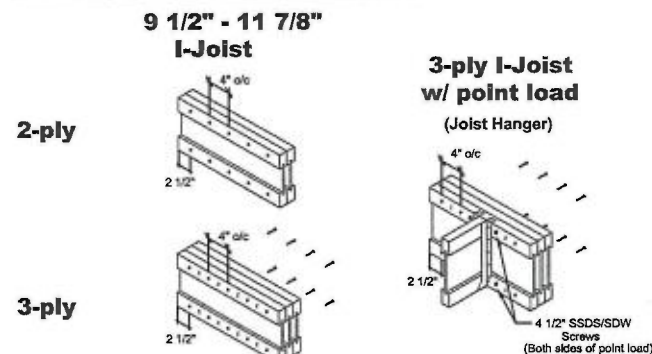


LVL connection notes:

- Nails to be 3 1/2" spiral wire nails.
- Nails to be located a minimum of 2 1/2" from the top and bottom of the member. Start all nails a minimum of 2 1/2" in from ends.
- Minimum 3 1/4" spacing between rows.
- Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail or screw driven from the opposite side.

For side-loaded 4-ply LVL Connections, please consult the engineering calculation page for the component and the Nascor layout

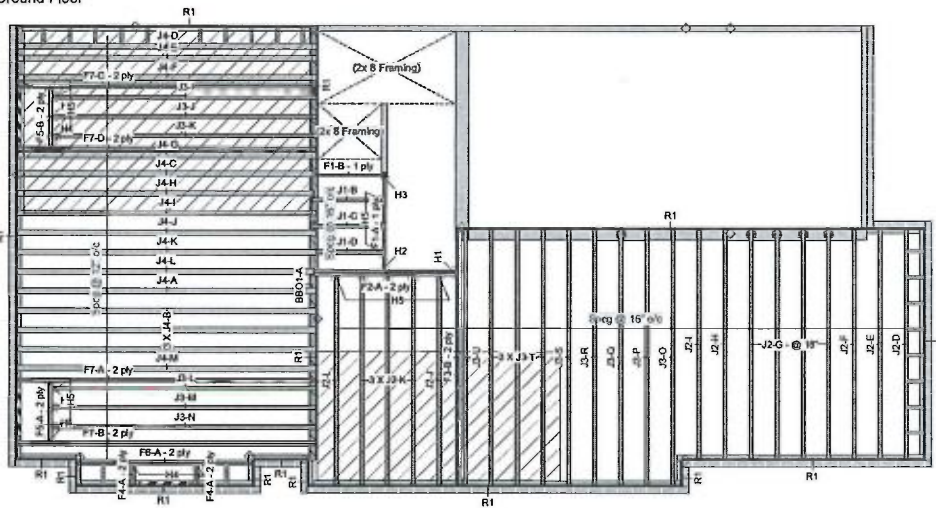
Vertical I-Joist Connections (for uniform distributed loads)



Vertical I-Joist connection notes:

- Nails to be 3" spiral wire nails.
- Nails to be located at centre of top and bottom flanges. Start all nails a minimum of 2 1/2" in from ends.
- Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail driven from the opposite side.

Ground Floor



This certification is to confirm that:

1. The loads used in the calculation of the attached approved components conform to the floor assembly shown on this layout.
2. The floor joists comply with the Nascor span table for the loads and spacing shown on this layout.

The floor system must be assembled in accordance to the Nascor Specifier Guide. Multi-ply members must be attached together as per the included multiple member connection detail.

All other components and structural elements supporting the floor system such as beams, walls, columns and foundation walls and footings including anchorage of components and bracing for lateral stability are the responsibility of others.



September 13, 2018

Legend

	Load from Above
	Wall
	Wall Opening
	Nasbord Rimboard Plus 1.125 X 9.5
	NJ 9.5
	NJ40U 9.5
	NJH 9.5
	Forex 2.0E-3000Fb LVL 1.75 X 9.5

1. CBC 2012 O Reg 332/12 as amended
2. Nascor CCMC - 13535-R
3. LVL CCMC - 14058-R
4. CAN/CSA-C88-09
5. CCMC-12787-RAPA PR-L310(C)

Ground Floor

LVL/LSL (Flush)

Label	Description	Width	Depth	Qty	Piles	Pcs	Length
F3	Forex 2.0E-3000Fb LVL	1.75	9.5	1	2	2	14-0-0
F2	Forex 2.0E-3000Fb LVL	1.75	9.5	1	2	2	8-0-0
F1	Forex 2.0E-3000Fb LVL	1.75	9.5			2	6-0-0

Joist (Flush)

Label	Description	Width	Depth	Qty	Piles	Pcs	Length
F7	NJ	1.5	9.5	4	2	8	16-0-0
F6	NJ	1.5	9.5	1	2	2	10-0-0
F5	NJ	1.5	9.5	2	2	4	4-0-0
F4	NJ	1.5	9.5	2	2	4	2-0-0
J4	NJ40U	3.5	9.5			15	16-0-0
J3	NJH	2.5	9.5			15	14-0-0
J2	NJH	2.5	9.5			14	12-0-0
J1	NJH	2.5	9.5			3	4-0-0

Rim Board

Label	Description	Width	Depth	Qty	Piles	Pcs	Length
R1	Nasbord Rimboard Plus 1.125 X 9.5	1.125	9.5			12	12

Blocking

Label	Description	Width	Depth	Qty	Piles	Pcs	Length
BLK1	NJH	2.5	9.5			Varies	16-0-0

Hanger

Label	Pcs	Description	Skew	Slope	fasteners	Supported Member
H1	1	HGUS410			46 16d	16 16d
H2	1	HUS1.61/10			30 16d	10 16d
H3	1	HUCQ1.61/9-SDS				
H4	6	LT2-169			4 10dx1 1/2	2 10dx1 1/2
H5	14	LT259			4 10dx1 1/2	2 10dx1 1/2

NOTES:

1. Framers to verify dimensions on the architectural drawings.
2. Double joist only require filler/backer ply when supporting another member using a face-mounted hanger.
3. Install 2x4 blocking @ 24" o/c under parallel non-load bearing walls.
4. Install single-ply flush window header along inside face of rimboard/rimjoist.
5. Refer to Nascor specifier guide for installation works.
6. Squash blocks recommended to be installed at end bearing on all first level joists which support loading from above exceeding two levels floor or roof.
7. Load transfer blocks to be installed under all point loads.
8. It shall be the framer's responsibility that floor joists and beams are fastened as per the hanger manufacturer's standards.

Refer to Multiple Member Connection Detail to ply to ply nailing or bolting requirements.

Rim parallel to joists: 1-1/8" rimboard with 2"x4" block (1/16" longer than rim depth @ 16" o/c). All other components and structural elements supporting the floor system such as beams, walls, columns, and foundation walls and footings including anchorage of components and bracing for lateral stability are the responsibility of Others.

Hatch area represents ceramic tiled floor with an additional dead load of 5 PSF

The framing shown on this layout may deviate from the architectural and structural drawings. Project Engineer to review and approve the deviation prior to construction.

ARCHITECTURAL DRAWINGS:

JARDIN DESIGN GROUP INC.
64 Jardin Dr., Suite 3A, Vaughan, ON
Date: Rev 3, Aug 30, 2018
Project No: 18-24
Model: Clover 11A



Layout Name

CLOVER 11A-1

Design Method

LSD

Description

MINNISALE HOMES

BRAMPTON, ONT.

Created

July 03, 2018

Builder

GREENPARK

Sales Rep

RM

Designer

RCO

Shipping

Project

Builder's Project

Kott Lumber Company

14 Anderson Blvd

Stouffville, Ontario

Canada

L4A 7X4

905-642-4400

Job Path

D:\Users\wchochawicz\WORK FROM

HOMES\GREENPARK\MINNISALE

HOMES\CLOVER 11A\FLOOR\REV

CLOVER 11A.rvt

Ground Floor

Design Method

LSD

Building Code

NBCC 2010 / CBC

2012

Floor

Loads

Live

40

Dead

15

Deflection Joist

LL Span 1/

480

TL Span 1/

360

LL Cant 2L/

480

TL Cant 2L/

360

Deflection Girder

LL Span 1/

360

TL Span 1/

240

LL Cant 2L/

480

TL Cant 2L/

360

Decking

Deck

SPF Plywood

Thickness

3/4"

Fastener

Nailed & Glued

Vibration



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Client: GREENPARK

Project:

Address:

Date: 9/7/2018

Designer: RCO

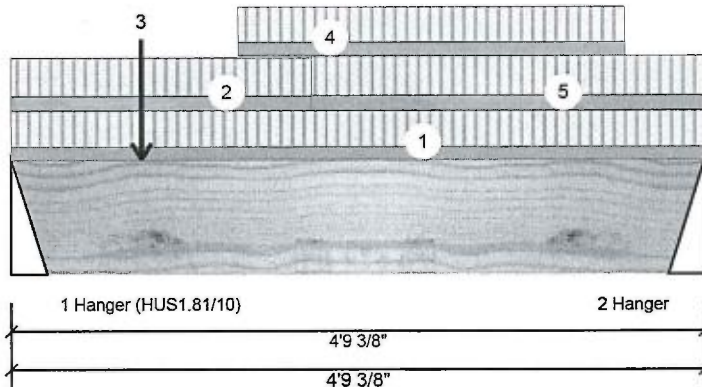
Job Name: CLOVER 11A-1

Project #:

Page 1 of 1

F1-A Forex 2.0E-3000Fb LVL 1.750" X 9.500" - PASSED

Level: Ground Floor

**Member Information****Unfactored Reactions UNPATTERNED lb (Uplift)**

Type:	Girder	Application:	Floor (Residential)
Plies:	1	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		

Brg	Live	Dead	Snow	Wind
1	497	196	0	0
2	487	191	0	0

Bearings and Factored Reactions

Bearing	Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
1 - Hanger	3.000"	25% 244 / 745	990 L	1.25D+1.5L
2 - Hanger	3.000"	25% 239 / 730	970 L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1048 ft-lb	2'4 3/4"	11362 ft-lb	0.092 (9%)	1.25D+1.5L	L
Unbraced	1048 ft-lb	2'4 3/4"	8295 ft-lb	0.126 (13%)	1.25D+1.5L	L
Shear	760 lb	11 3/4"	4638 lb	0.164 (16%)	1.25D+1.5L	L
Perm Defl in. (L/12248)	0.004	2'4 11/16"	0.147 (L/360)	0.030 (3%)	D	Uniform
LL Defl inch	0.011 (L/4802)	2'4 11/16"	0.147 (L/360)	0.070 (7%)	L	L
TL Defl inch	0.015 (L/3449)	2'4 11/16"	0.220 (L/240)	0.070 (7%)	D+L	L

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.
- 4 Bottom braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 4-9-6	(Span)3-6-8	Top	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 2-0-14	(Span)3-10-0	Top	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	0-10-13		Far Face	32 lb	84 lb	0 lb	0 lb	J1
4	Part. Uniform	1-6-13 to 4-2-13		Far Face	26 PLF	70 PLF	0 PLF	0 PLF	
5	Tie-In	2-0-14 to 4-9-6	(Span)3-11-12	Top	15 PSF	40 PSF	0 PSF	0 PSF	
	Self Weight				4 PLF				



September 13, 2018

PASSE Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

Calculated Structural Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

1. Dry service conditions, unless noted otherwise
2. LVL not to be treated with fire retardant or corrosive

chemicals**Handling & Installation**

1. LVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used
4. Design assumes top edge is laterally restrained
5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex
APA: PR-L318

Kott Lumber Company
14 Anderson Blvd, Ontario
Canada
L4A 7X4
905-642-4400

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

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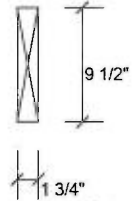
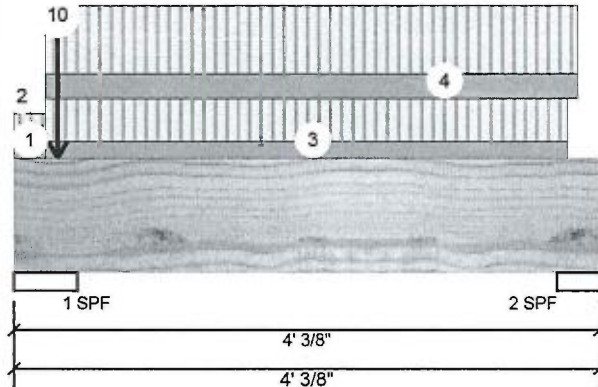
Client: GREENPARK
 Project:
 Address:

Date: 9/7/2018
 Designer: RCO
 Job Name: CLOVER 11A-1
 Project #:

Page 1 of 2

F1-B Forex 2.0E-3000Fb LVL 1.750" X 9.500" - PASSED

Level: Ground Floor


Member Information
Unfactored Reactions UNPATTERNED lb (Uplift)

Type:	Girder	Application:	Floor (Residential)
Plies:	1	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		

Brg	Live	Dead	Snow	Wind
1	1058	586	0	0
2	116	51	0	0

Bearings and Factored Reactions

Bearing	Length	Cap. React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	5.250"	51%	732 / 1587	2319 L	1.25D+1.5L
2 - SPF	3.500"	6%	64 / 174	238 L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	197 ft-lb	2'1 1/16"	11362 ft-lb	0.017 (2%)	1.25D+1.5L	L
Unbraced	197 ft-lb	2'1 1/16"	9506 ft-lb	0.021 (2%)	1.25D+1.5L	L
Shear	124 lb	1'2"	4638 lb	0.027 (3%)	1.25D+1.5L	L
Perm Defl in.	0.001 (L/63986)	2'1 1/8"	0.114 (L/360)	0.010 (1%)	D	Uniform
LL Defl inch	0.001 (L/27704)	2'1 1/8"	0.114 (L/360)	0.010 (1%)	L	L
TL Defl inch	0.002 (L/19333)	2'1 1/8"	0.171 (L/240)	0.010 (1%)	D+L	L

Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.
- 4 Bottom braced at bearings.



September 13, 2018

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 0-2-10	(Span)0-11-7	Top	15 PSF	40 PSF	0 PSF	0 PSF	
2	Part. Uniform	0-0-0 to 0-1-6		Top	2 PLF	0 PLF	0 PLF	0 PLF	
3	Tie-In	0-2-10 to 3-9-12	(Span)1-3-7	Top	15 PSF	40 PSF	0 PSF	0 PSF	
4	Part. Uniform	0-2-10 to 3-10-10		Top	15 PLF	40 PLF	0 PLF	0 PLF	
5	Point	0-3-10		Top	382 lb	749 lb	0 lb	0 lb	F10 F10
6	Point	0-3-10		Top	1 lb	2 lb	0 lb	0 lb	J1

Continued on page 2...

Notes

Calculated Structural Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

1. Dry service conditions, unless noted otherwise
2. LVL not to be treated with fire retardant or corrosive

chemicals
Handling & Installation

1. LVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used
4. Design assumes top edge is laterally restrained
5. Provide lateral support at bearing points to avoid lateral displacement and rotation

5. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex
 APA: PR-L318

Kott Lumber Company
 14 Anderson Blvd, Ontario
 Canada
 L4A 7X4
 905-642-4400

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This design

NASCOR




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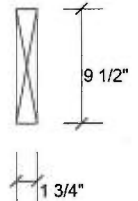
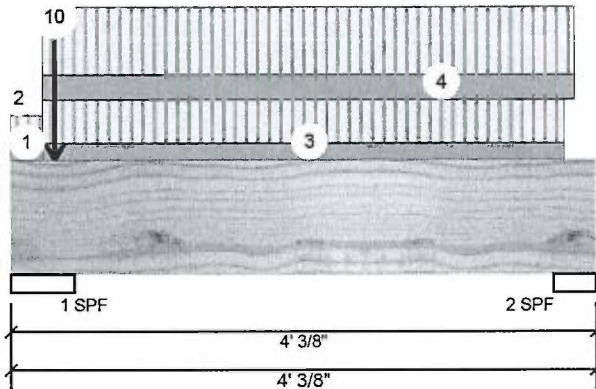
Client: GREENPARK
 Project:
 Address:

Date: 9/7/2018
 Designer: RCO
 Job Name: CLOVER 11A-1
 Project #:

Page 2 of 2

F1-B Forex 2.0E-3000Fb LVL 1.750" X 9.500" - PASSED

Level: Ground Floor



Continued from page 1

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
7	Point	0-3-10		Top	51 lb	129 lb	0 lb	0 lb	J6
8	Point	0-3-10		Top	57 lb	0 lb	0 lb	0 lb	Wall Self Weight
9	Point	0-3-10		Top	21 lb	51 lb	0 lb	0 lb	J6
10	Point	0-3-10		Top	18 lb	0 lb	0 lb	0 lb	Wall Self Weight
	Self Weight				4 PLF				

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

Calculated Structural Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

1. Dry service conditions, unless noted otherwise
2. LVL not to be treated with fire retardant or corrosive chemicals

chemicals**Handling & Installation**

1. LVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used
4. Design assumes top edge is laterally restrained
5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex
 APA: PR-L318

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 14 Anderson Blvd, Ontario
 Canada
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NASCOR

This design is valid until 7/10/2021





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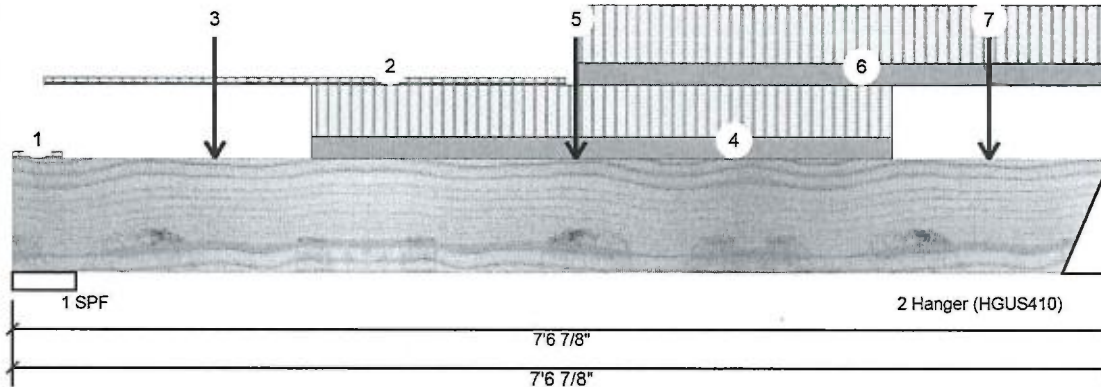
Client: GREENPARK
Project:
Address:

Date: 9/7/2018
Designer: RCO
Job Name: CLOVER 11A-1
Project #:

Page 1 of 2

F2-A Forex 2.0E-3000Fb LVL 1.750" X 9.500" 2-Ply - PASSED

Level: Ground Floor



Member Information

Type:	Girder	Application:	Floor (Residential)
Piles:	2	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	1147	493	0	0
2	1674	693	0	0

Bearings and Factored Reactions

Bearing	Length	Cap. React	D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	5.250"	21%	617 / 1720	2337	L	1.25D+1.5L
2 - Hanger	4.000"	32%	866 / 2512	3378	L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	5818 ft-lb	3'10 5/8"	22724 ft-lb	0.256 (26%)	1.25D+1.5L	L
Unbraced	5818 ft-lb	3'10 5/8"	21776 ft-lb	0.267 (27%)	1.25D+1.5L	L
Shear	2866 lb	6'6 1/8"	9277 lb	0.309 (31%)	1.25D+1.5L	L
Perm Defl in.	0.024 (L/3517)	3'10 5/8"	0.231 (L/360)	0.100 (10%)	D	Uniform
LL Defl inch	0.057 (L/1467)	3'10 5/8"	0.231 (L/360)	0.250 (25%)	L	L
TL Defl inch	0.080 (L/1035)	3'10 5/8"	0.346 (L/240)	0.230 (23%)	D+L	L

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on full section width.



September 13, 2018

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 0-4-2	(Span)0-11-7	Top	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-2-10 to 3-9-12	(Span)1-0-9	Top	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	1-4-12		Near Face	107 lb	251 lb	0 lb	0 lb	J2
4	Part. Uniform	2-0-12 to 6-0-12		Near Face	90 PLF	215 PLF	0 PLF	0 PLF	
5	Point	3-10-10		Far Face	196 lb	497 lb	0 lb	0 lb	F1
6	Part. Uniform	3-10-12 to 7-6-14		Top	90 PLF	240 PLF	0 PLF	0 PLF	

Continued on page 2...

Notes

Calculated Structural Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

1. Dry service conditions, unless noted otherwise
2. LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

1. LVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used
4. Design assumes top edge is laterally restrained
5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex
APA: PR-L318

Kott Lumber Company
14 Anderson Blvd, Ontario
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L4A 7X4
905-642-4400

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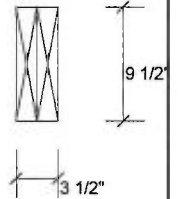
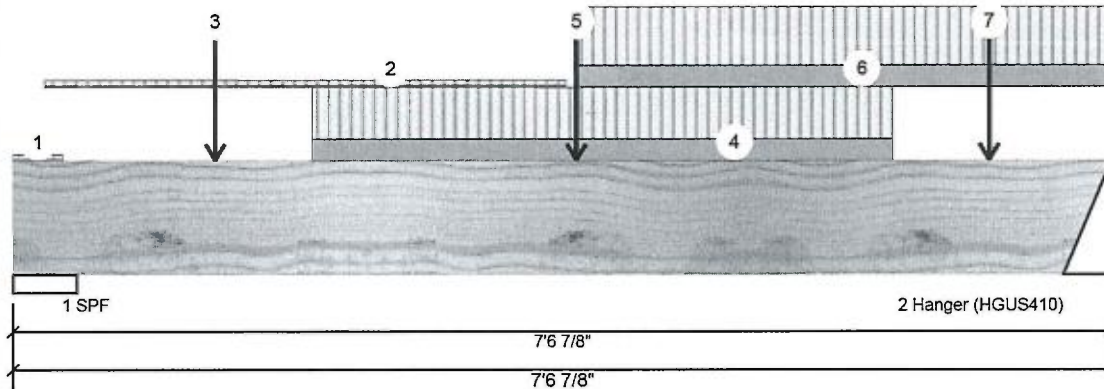
Client: GREENPARK
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 Project #:

Page 2 of 2

F2-A Forex 2.0E-3000Fb LVL 1.750" X 9.500" 2-Ply - PASSED

Level: Ground Floor



...Continued from page 1

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
7	Point	6-8-12		Near Face	104 lb	249 lb	0 lb	0 lb	J2
	Self Weight				8 PLF				

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

Calculated Structural Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

1. Dry service conditions, unless noted otherwise
2. LVL not to be treated with fire retardant or corrosive

chemicals**Handling & Installation**

1. LVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used
4. Design assumes top edge is laterally restrained
5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex
 APA: PR-L318

Kott Lumber Company
 14 Anderson Blvd, Ontario
 Canada
 L4A 7X4
 905-642-4400

NASCOR



This design is valid until 7/10/2021



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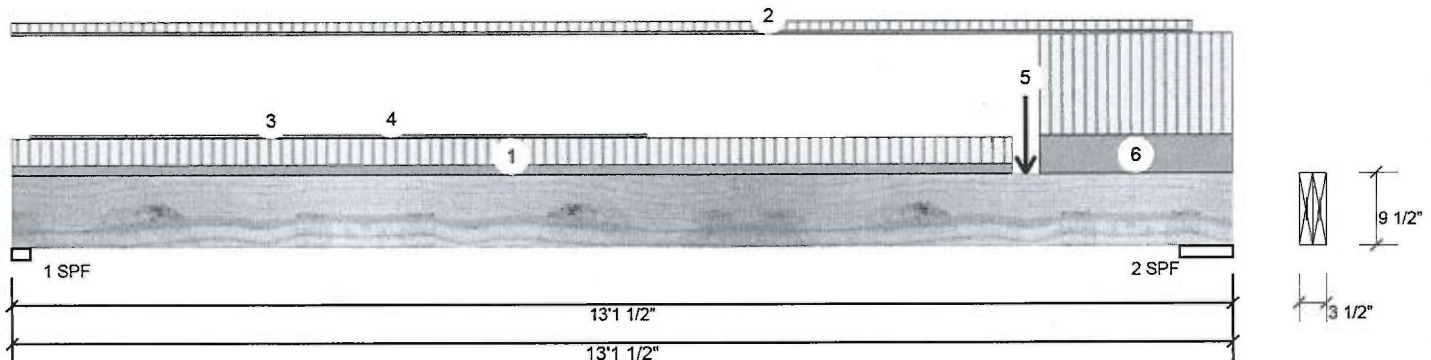
Client: GREENPARK
Project:
Address:

Date: 9/7/2018
Designer: RCO
Job Name: CLOVER 11A-1
Project #:

Page 1 of 1

F3-B Forex 2.0E-3000Fb LVL 1.750" X 9.500" 2-Ply - PASSED

Level: Ground Floor



Member Information

Unfactored Reactions UNPATTERNED lb (Uplift)

Type:	Girder	Application:	Floor (Residential)
Ply:	2	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		

Brg	Live	Dead	Snow	Wind
1	404	223	0	0
2	1729	762	0	0

Bearings and Factored Reactions

Bearing	Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
1 - SPF	2.375"	17%	279 / 606	885 L 1.25D+1.5L
2 - SPF	6.875"	24%	952 / 2594	3546 L 1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	5663 ft-lb	10'10 7/8"	22724 ft-lb	0.249 (25%)	1.25D+1.5L	L
Unbraced	5663 ft-lb	10'10 7/8"	19648 ft-lb	0.288 (29%)	1.25D+1.5L	L
Shear	3325 lb	11'9 7/8"	9277 lb	0.358 (36%)	1.25D+1.5L	L
Perm Defl in.	0.065 (L/2313)	7' 3/16"	0.416 (L/360)	0.160 (16%)	D	Uniform
LL Defl inch	0.134 (L/1118)	7'1 7/8"	0.416 (L/360)	0.320 (32%)	L	L
TL Defl inch	0.199 (L/754)	7'1 3/8"	0.624 (L/240)	0.320 (32%)	D+L	L

Design Notes

- Girders are designed to be supported on the bottom edge only.
- Multiple plies must be fastened together as per manufacturer's details.
- Top loads must be supported equally by all plies.
- Top braced at bearings.
- Bottom braced at bearings.
- Lateral slenderness ratio based on full section width.



September 13, 2018

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 10-9-2	(Span) 0-11-14	Top	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 12-8-6	(Span) 0-4-2	Top	15 PSF	40 PSF	0 PSF	0 PSF	
3	Part. Uniform	0-2-7 to 6-10-0		Top	1 PLF	0 PLF	0 PLF	0 PLF	
4	Part. Uniform	0-2-7 to 6-10-0		Top	2 PLF	0 PLF	0 PLF	0 PLF	
5	Point	10-10-14		Far Face	693 lb	1674 lb	0 lb	0 lb	Pass Thru Framing Squash Block is required at all point loads over bearings
6	Tie-In	11-0-10 to 13-1-8	(Span) 3-10-0	Top	15 PSF	40 PSF	0 PSF	0 PSF	Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements
	Self Weight				8 PLF				

Notes

Calculated Structural Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

- Dry service conditions, unless noted otherwise
- LVL not to be treated with fire retardant or corrosive chemicals

chemicals

Handling & Installation

- LVL beams must not be cut or drilled
- Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained
- Provide lateral support at bearing points to avoid lateral displacement and rotation

- For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex
APA: PR-L318

Kott Lumber Company
14 Anderson Blvd, Ontario
Canada
L4A 7X4
905-642-4400

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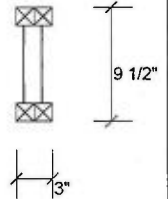
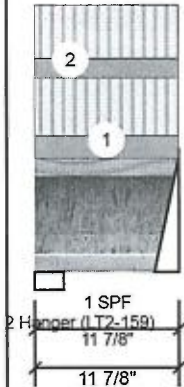
Client: GREENPARK
Project:
Address:

Date: 9/7/2018
Designer: RCO
Job Name: CLOVER 11A-1
Project #:

Page 1 of 1

F4-A NJ 9.500" 2-Ply - PASSED

Level: Ground Floor



Member Information

Unfactored Reactions UNPATTERNED lb (Uplift)

Type: Girder	Application: Floor (Residential)	Brg	Live	Dead	Snow	Wind
Piles: 2	Design Method: LSD	1	63	24	0	0
Moisture Condition: Dry	Building Code: NBCC 2010 / OBC 2012	2	59	22	0	0
Deflection LL: 360	Load Sharing: No					
Deflection TL: 240	Deck: Not Checked					
Importance: Normal	Vibration: Not Checked					
General Load						
Floor Live: 40 PSF						
Dead: 15 PSF						

Bearings and Factored Reactions

Bearing	Length	Cap. React	D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	2.375"	5%	29 / 94	124	L	1.25D+1.5L
2 - Hanger	2.000"	4%	28 / 89	116	L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	17 ft-lb	6 1/8"	7340 ft-lb	0.002 (0%)	1.25D+1.5L	L
Unbraced	17 ft-lb	6 1/8"	7260 ft-lb	0.002 (0%)	1.25D+1.5L	L
Shear	91 lb	10 5/8"	3080 lb	0.030 (3%)	1.25D+1.5L	L
Perm Defl in.	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
LL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
TL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top flange braced at bearings.
- 6 Bottom flange braced at bearings.



September 13, 2018

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 0-11-14	(Span)3-3-0	Top	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 0-11-14	(Span)2-10-15	Top	15 PSF	40 PSF	0 PSF	0 PSF	

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

Calculated Structural Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

1. Dry service conditions, unless noted otherwise
2. Ljoist not to be treated with fire retardant or corrosive chemicals

chemicals

Handling & Installation

1. Ljoist flanges must not be cut or drilled
2. Refer to latest copy of the Ljoist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-ply fastening details and handling/instruction details.
3. Damaged Ljoists must not be used
4. Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

5. Provide lateral support at bearing points to avoid lateral displacement and rotation
6. Web stiffeners for point load as shown Minimum point load bearing length= 3.5 inches
7. For flat roofs provide ponding

Manufacturer Info

Nascor by Kott

Kott Lumber Company
14 Anderson Blvd, Ontario
Canada
L4A 7X4
905-642-4400

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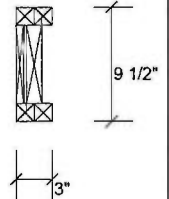
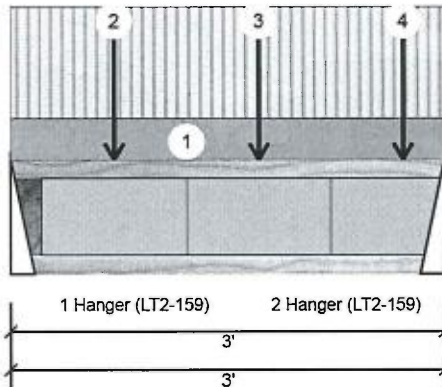
Client: GREENPARK
 Project:
 Address:

Date: 9/7/2018
 Designer: RCO
 Job Name: CLOVER 11A-1
 Project #:

Page 1 of 1

F5-A NJ 9.500" 2-Ply - PASSED

Level: Ground Floor



Member Information

Type:	Girder	Application:	Floor (Residential)
Plies:	2	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	370	139	0	0
2	437	164	0	0

Bearings and Factored Reactions

Bearing	Length	Cap. React D/L lb	Total	Ld. Case	Ld. Comb.
1 - Hanger	2.000"	28%	173 / 555	728 L	1.25D+1.5L
2 - Hanger	2.000"	33%	205 / 655	860 L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	587 ft-lb	1'8 9/16"	7340 ft-lb	0.080 (8%)	1.25D+1.5L	L
Unbraced	587 ft-lb	1'8 9/16"	4678 ft-lb	0.126 (13%)	1.25D+1.5L	L
Shear	852 lb	2'10 3/4"	3080 lb	0.277 (28%)	1.25D+1.5L	L
Perm Defl in.	0.002 (L/19612)	1'8 9/16"	0.093 (L/360)	0.020 (2%)	D	Uniform
LL Defl inch	0.005 (L/7346)	1'8 9/16"	0.093 (L/360)	0.050 (5%)	L	L
TL Defl inch	0.006 (L/5344)	1'8 9/16"	0.140 (L/240)	0.040 (4%)	D+L	L

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top flange braced at bearings.
- 6 Bottom flange braced at bearings.



September 13, 2018

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 3-0-0	(Span)1-8-15	Top	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	0-8-9		Near Face	92 lb	246 lb	0 lb	0 lb	J3
3	Point	1-8-9		Near Face	100 lb	267 lb	0 lb	0 lb	J3
4	Point	2-8-9		Near Face	71 lb	189 lb	0 lb	0 lb	J3

Pass Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

Calculated Structural Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

1. Dry service conditions, unless noted otherwise
2. Ljoist not to be treated with fire retardant or corrosive chemicals

chemicals

Handling & Installation

1. Ljoist flanges must not be cut or drilled
2. Refer to latest copy of the Ljoist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-ply fastening details and handling/eraction details
3. Damaged Ljoists must not be used
4. Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

5. Provide lateral support at bearing points to avoid lateral displacement and rotation
6. Web stiffeners for point load as shown Minimum point load bearing length ≥ 3.5 inches
7. For flat roofs provide ponding

Manufacturer Info

Nascor by Kott

Kott Lumber Company
 14 Anderson Blvd, Ontario
 Canada
 L4A 7X4
 905-642-4400

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Client: GREENPARK

Project:

Address:

Date: 9/7/2018

Designer: RCO

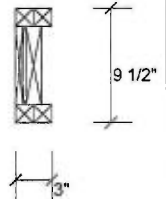
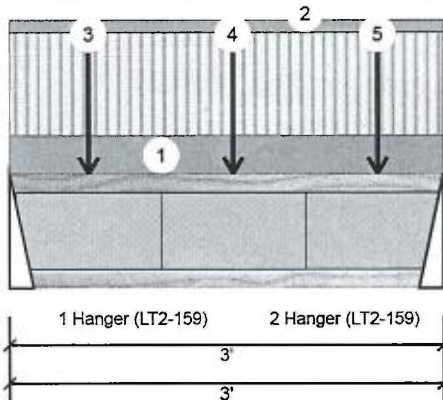
Job Name: CLOVER 11A-1

Project #:

Page 1 of 1

F5-B NJ 9.500" 2-Ply - PASSED

Level: Ground Floor

**Member Information****Unfactored Reactions UNPATTERNED lb (Uplift)**

Type:	Girder	Application:	Floor (Residential)
Plies:	2	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		

Brg	Live	Dead	Snow	Wind
1	395	193	0	0
2	410	195	0	0

Bearings and Factored Reactions

Bearing	Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
1 - Hanger	2.000"	32% 241 / 593	834 L	1.25D+1.5L
2 - Hanger	2.000"	33% 244 / 615	860 L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	642 ft-lb	1'6 9/16"	7340 ft-lb	0.087 (9%)	1.25D+1.5L	L
Unbraced	642 ft-lb	1'6 9/16"	4678 ft-lb	0.137 (14%)	1.25D+1.5L	L
Shear	852 lb	2'10 3/4"	3080 lb	0.277 (28%)	1.25D+1.5L	L
Perm Defl in.	0.002 (L/14913)	1'6 9/16"	0.093 (L/360)	0.020 (2%)	D	Uniform
LL Defl inch	0.005 (L/7160)	1'6 9/16"	0.093 (L/360)	0.050 (5%)	L	L
TL Defl inch	0.007 (L/4838)	1'6 9/16"	0.140 (L/240)	0.050 (5%)	D+L	L

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top flange braced at bearings.
- 6 Bottom flange braced at bearings.



September 13, 2018

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-in	0-0-0 to 3-0-0	(Span)1-8-15	Top	15 PSF	40 PSF	0 PSF	0 PSF	
2	Part. Uniform	0-0-0 to 3-0-0		Top	4 PLF	0 PLF	0 PLF	0 PLF	
3	Point	0-6-9		Near Face	111 lb	223 lb	0 lb	0 lb	J3
4	Point	1-6-9		Near Face	127 lb	267 lb	0 lb	0 lb	J3
5	Point	2-6-9		Near Face	99 lb	211 lb	0 lb	0 lb	J3

Pass Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

Calculated Structural Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

1. Dry service conditions, unless noted otherwise
2. Joist not to be treated with fire retardant or corrosive chemicals

chemicals**Handling & Installation**

1. Joist flanges must not be cut or drilled
2. Refer to latest copy of the Joist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-ply fastening details and handling/installation details
3. Damaged Joists must not be used
4. Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. Web stiffeners for point load as shown Minimum point load bearing length ≥ 3.5 inches
7. For flat roofs provide ponding

Manufacturer Info

Nascor by Kott

Kott Lumber Company
14 Anderson Blvd, Ontario
Canada
L4A 7X4
905-642-4400

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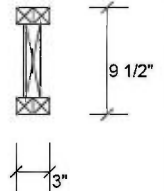
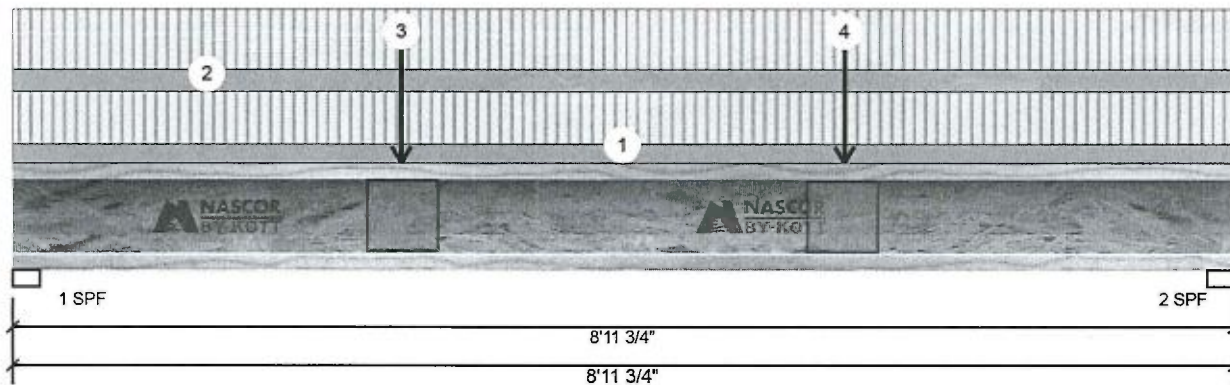
Client: GREENPARK
Project:
Address:

Date: 9/7/2018
Designer: RCO
Job Name: CLOVER 11A-1
Project #:

Page 1 of 1

F6-A NJ 9.500" 2-Ply - PASSED

Level: Ground Floor



Member Information

Type:	Girder	Application:	Floor (Residential)
Piles:	2	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	253	95	0	0
2	253	95	0	0

Bearings and Factored Reactions

Bearing	Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
1 - SPF	2.375"	19%	118 / 380	498 L 1.25D+1.5L
2 - SPF	2.375"	19%	118 / 380	498 L 1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1123 ft-lb	4'5 7/8"	7340 ft-lb	0.153 (15%)	1.25D+1.5L	L
Unbraced	1123 ft-lb	4'5 7/8"	1134 ft-lb	0.991 (99%)	1.25D+1.5L	L
Shear	487 lb	1 5/8"	3080 lb	0.158 (16%)	1.25D+1.5L	L
Perm Defl in.	0.013 (L/8149)	4'5 7/8"	0.290 (L/360)	0.040 (4%)	D	Uniform
LL Defl inch	0.034 (L/3051)	4'5 7/8"	0.290 (L/360)	0.120 (12%)	L	L
TL Defl inch	0.047 (L/2220)	4'5 7/8"	0.435 (L/240)	0.110 (11%)	D+L	L

Design Notes

- Girders are designed to be supported on the bottom edge only.
- Multiple plies must be fastened together as per manufacturer's details.
- Top loads must be supported equally by all plies.
- Top flange must be laterally braced at a maximum of 5'11" o.c.
- Bottom flange braced at bearings.



September 13, 2018

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 8-11-12	(Span)1-0-0	Top	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 8-11-12	(Span)1-1-15	Top	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	2-10-6		Near Face	22 lb	59 lb	0 lb	0 lb	F4
4	Point	6-1-6		Near Face	22 lb	59 lb	0 lb	0 lb	F4

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

Calculated Structural Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

- Dry service conditions, unless noted otherwise
- Ljoist not to be treated with fire retardant or corrosive chemicals

chemicals

Handling & Installation

- Ljoist flanges must not be cut or drilled
- Refer to latest copy of the Ljoist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-ply fastening details and handling/installation details
- Damaged Ljoists must not be used
- Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
- Web stiffeners for point load as shown Minimum point load bearing length = 3.5 inches
- For flat roofs, provide ponding

Manufacturer Info

Nascor by Kott

Kott Lumber Company
14 Anderson Blvd, Ontario
Canada
L4A 7X4
905-642-4400

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This design



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Client: GREENPARK

Project:

Address:

Date: 9/7/2018

Designer: RCO

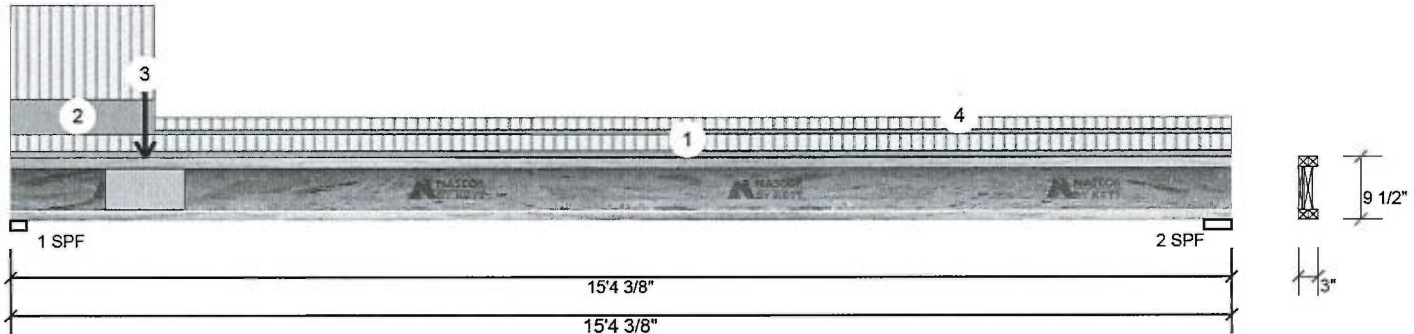
Job Name: CLOVER 11A-1

Project #:

Page 1 of 1

F7-A NJ 9.500" 2-Ply - PASSED

Level: Ground Floor

**Member Information****Unfactored Reactions UNPATTERNED lb (Uplift)**

Type:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind
Plies:	2	Design Method:	LSD	1	642	241	0	0
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012	2	206	77	0	0
Deflection LL:	360	Load Sharing:	No					
Deflection TL:	240	Deck:	Not Checked					
Importance:	Normal	Vibration:	Not Checked					
General Load								
Floor Live:	40 PSF							
Dead:	15 PSF							
				Bearings and Factored Reactions				
				Bearing	Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.

Bearings and Factored Reactions

Bearing	Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
1 - SPF	2.375"	47%	301 / 962	1263 L 1.25D+1.5L
2 - SPF	4.125"	13%	97 / 309	406 L 1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1980 ft-lb	5' 11/16"	7340 ft-lb	0.270 (27%)	1.25D+1.5L	L
Unbraced	1980 ft-lb	5' 11/16"	1987 ft-lb	0.996 (100%)	1.25D+1.5L	L
Shear	1243 lb	1 5/8"	3080 lb	0.404 (40%)	1.25D+1.5L	L
Perm Defl in.	0.059 (L/3063)	7' 11/16"	0.498 (L/360)	0.120 (12%)	D	Uniform
LL Defl inch	0.156 (L/1149)	7' 11/16"	0.498 (L/360)	0.310 (31%)	L	L
TL Defl inch	0.215 (L/836)	7' 11/16"	0.747 (L/240)	0.290 (29%)	D+L	L

Design Notes

- Girders are designed to be supported on the bottom edge only.
- Multiple plies must be fastened together as per manufacturer's details.
- Top loads must be supported equally by all plies.
- Top flange must be laterally braced at a maximum of 4'8" o.c.
- Bottom flange braced at bearings.



September 13, 2018

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 15-4-6	(Span)0-7-1	Top	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 1-9-14	(Span)3-3-0	Top	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	1-8-6		Near Face	164 lb	437 lb	0 lb	0 lb	F5
4	Tie-In	1-9-14 to 15-4-6	(Span)0-4-15	Top	15 PSF	40 PSF	0 PSF	0 PSF	

0 PSF
Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

Calculated Structural Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

- Dry service conditions, unless noted otherwise
- Joist not to be treated with fire retardant or corrosive chemicals

chemicals**Handling & Installation**

- Joist flanges must not be cut or drilled
- Refer to latest copy of the Joist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-ply fastening details and handling/erection details
- Damaged Joists must not be used
- Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
- Web stiffeners for point load as shown Minimum point load bearing length >= 3.5 inches
- For flat roofs provide ponding

Manufacturer Info

Nascor by Kott

Kott Lumber Company
14 Anderson Blvd, Ontario
Canada
L4A 7X4
905-642-4400

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

This design is v

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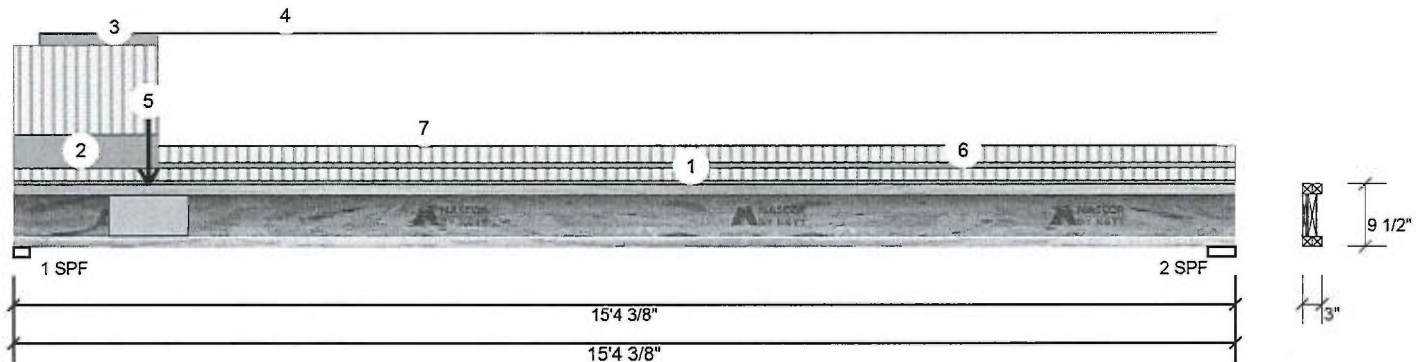
Client: GREENPARK
Project:
Address:

Date: 9/7/2018
Designer: RCO
Job Name: CLOVER 11A-1
Project #:

Page 1 of 1

F7-C NJ 9.500" 2-Ply - PASSED

Level: Ground Floor



Member Information

Type:	Girder	Application:	Floor (Residential)
Plies:	2	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		

Unfactored Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	Wind
1	612	291	0	0
2	203	96	0	0

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	2.375"	48%	363 / 917	1281	L	1.25D+1.5L
2 - SPF	4.125"	14%	120 / 305	425	L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2042 ft-lb	5'2 1/2"	7340 ft-lb	0.278 (28%)	1.25D+1.5L	L
Unbraced	2042 ft-lb	5'2 1/2"	2046 ft-lb	0.998 (100%)	1.25D+1.5L	L
Shear	1261 lb	1 5/8"	3080 lb	0.410 (41%)	1.25D+1.5L	L
Perm Defl in.	0.072 (L/2480)	7' 7/8"	0.498 (L/360)	0.150 (15%)	D	Uniform
LL Defl inch	0.152 (L/1182)	7'1"	0.498 (L/360)	0.300 (30%)	L	L
TL Defl inch	0.224 (L/800)	7'1"	0.747 (L/240)	0.300 (30%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 4'8" o.c.
- 5 Bottom flange braced at bearings.



September 13, 2018

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 15-4-6	(Span)0-5-1	Top	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 1-9-14	(Span)3-3-0	Top	15 PSF	40 PSF	0 PSF	0 PSF	
3	Part. Uniform	0-3-14 to 1-9-14		Top	8 PLF	0 PLF	0 PLF	0 PLF	
4	Part. Uniform	0-3-14 to 15-1-8		Top	1 PLF	0 PLF	0 PLF	0 PLF	
5	Point	1-8-6		Near Face	195 lb	410 lb	0 lb	0 lb	Pass-Thru Framing Squash Block is required at all point loads over bearings
6	Tie-In	1-9-14 to 15-4-6	(Span)0-6-15	Top	15 PSF	40 PSF	0 PSF	0 PSF	
7	Part. Uniform	1-9-14 to 15-1-9		Top	1 PLF	0 PLF	0 PLF	0 PLF	Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

1. Dry service conditions, unless noted otherwise
2. Joist not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

1. Joist flanges must not be cut or drilled
2. Refer to latest copy of the Joist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-ply fastening details and handling/erection details
3. Damaged Joists must not be used
4. Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

5. Provide lateral support at bearing points to avoid lateral displacement and rotation
 6. Web stiffeners for point load as shown Minimum point load bearing length ≥ 3.5 inches
 7. For flat roofs provide ponding
- READ ALL NOTES ON**

Manufacturer Info

Nascor by Kott

Kott Lumber Company
14 Anderson Blvd, Ontario
Canada
L4A 7X4
905-642-4400

**READ ALL NOTES ON THIS PAGE AND ON THE
ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE
IS AN INTEGRAL PART OF THIS DRAWING AS IT
CONTAINS SPECIFICATIONS AND CRITERIA USED
IN THE DESIGN OF THIS COMPONENT.**

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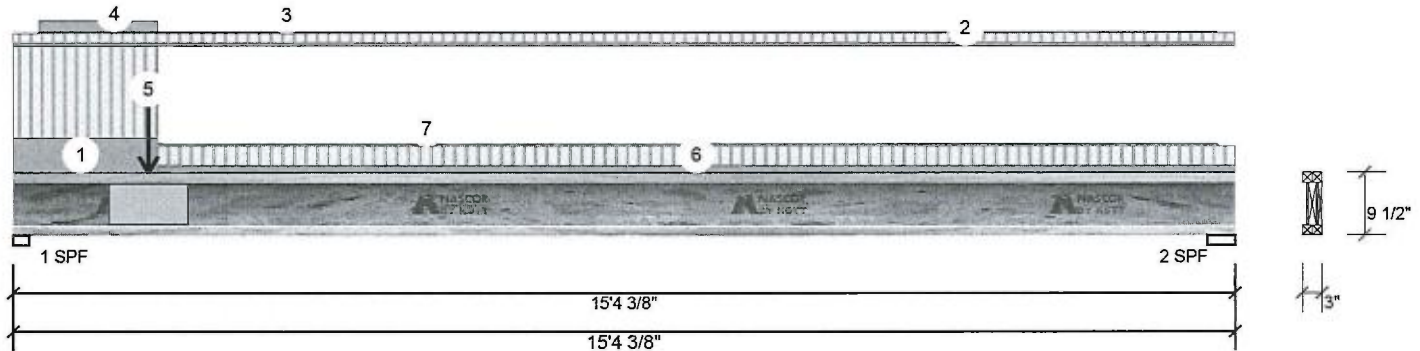
Client: GREENPARK
 Project:
 Address:

Date: 9/7/2018
 Designer: RCO
 Job Name: CLOVER 11A-1
 Project #:

Page 1 of 1

F7-D NJ 9.500" 2-Ply - PASSED

Level: Ground Floor



Member Information

Unfactored Reactions UNPATTERNED lb (Uplift)

Type:	Girder	Application:	Floor (Residential)
Piles:	2	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		

Brg	Live	Dead	Snow	Wind
1	595	294	0	0
2	201	103	0	0

Bearings and Factored Reactions

Bearing	Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
1 - SPF	2.375"	47%	367 / 892	1259 L 1.25D+1.5L
2 - SPF	4.125"	14%	129 / 302	431 L 1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2044 ft-lb	5'4 3/16"	7340 ft-lb	0.278 (28%)	1.25D+1.5L	L
Unbraced	2044 ft-lb	5'4 3/16"	2046 ft-lb	0.999 (100%)	1.25D+1.5L	L
Shear	1240 lb	1 5/8"	3080 lb	0.403 (40%)	1.25D+1.5L	L
Perm Defl in.	0.076 (L/2357)	7'1 3/8"	0.498 (L/360)	0.150 (15%)	D	Uniform
LL Defl inch	0.149 (L/1201)	7'1 1/8"	0.498 (L/360)	0.300 (30%)	L	L
TL Defl inch	0.225 (L/796)	7'1 3/16"	0.747 (L/240)	0.300 (30%)	D+L	L

Design Notes

- Girders are designed to be supported on the bottom edge only.
- Multiple plies must be fastened together as per manufacturer's details.
- Top loads must be supported equally by all plies.
- Top flange must be laterally braced at a maximum of 4'8" o.c.
- Bottom flange braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-9-14	(Span)3-3-0	Top	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 15-4-6	(Span)0-3-15	Top	15 PSF	40 PSF	0 PSF	0 PSF	
3	Part. Uniform	0-3-14 to 15-1-10		Top	1 PLF	0 PLF	0 PLF	0 PLF	
4	Part. Uniform	0-3-14 to 1-9-14		Top	8 PLF	0 PLF	0 PLF	0 PLF	
5	Point	1-8-6		Far Face	193 lb	395 lb	0 lb	0 lb	Pass-Thru Framing Squash Block is required at all point loads over bearings
6	Tie-In	1-9-14 to 15-4-6	(Span)0-8-1	Top	15 PSF	40 PSF	0 PSF	0 PSF	
7	Part. Uniform	1-9-14 to 15-1-10		Top	2 PLF	0 PLF	0 PLF	0 PLF	Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

Calculated Structural Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

- Dry service conditions, unless noted otherwise
- Lioist not to be treated with fire retardant or corrosive chemicals

chemicals

Handling & Installation

- Lioist flanges must not be cut or drilled
- Refer to latest copy of the Lioist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-ply fastening details and handling/installation details
- Damaged Lioists must not be used
- Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
- Web stiffeners for point load as shown Minimum point load bearing length >= 3.5 inches
- For flat roofs provide ponding

Manufacturer Info

Nascor by Kott

Kott Lumber Company
 14 Anderson Blvd, Ontario
 Canada
 L4A 7X4
 905-642-4400

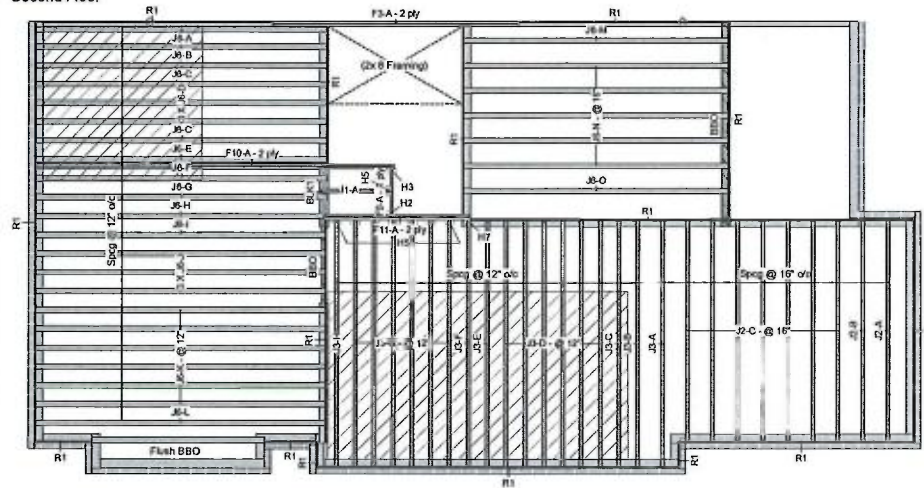
READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

This design is v

NASCOR



Second Floor



This certification is to confirm that:

1. The loads used in the calculation of the attached approved components conform to the floor assembly shown on this layout.
2. The floor joists comply with the Nascor span table for the loads and spacing shown on this layout.

The floor system must be assembled in accordance to the Nascor Specifier Guide. Multi-ply members must be attached together as per the included multiple member connection detail.

All other components and structural elements supporting the floor system such as beams, walls, columns and foundation walls and footings including anchorage of components and bracing for lateral stability are the responsibility of others.



September 13, 2018

Legend



Load from Above
Wall
Norbord Rimboard Plus 1.125 X 9.5
NJH 9.5
Forex 2.0E-3000Fb LVL 1.75 X 9.5

1. OBC 2012 O.Reg 332/12 as amended
2. Nascor CCMC - 13535-R
3. LVL CCMC - 14056-R
4. CAN/CSA-O86-09
5. CCMC - 12787-R APA PR-L310(C)

Second Floor

LVL/LSL (Flush)

Label	Description	Width	Depth	Qty	Piles	Pcs	Length
F10	Forex 2.0E-3000Fb LVL	1.75	9.5	1	2	2	20'-0"
F3	Forex 2.0E-3000Fb LVL	1.75	9.5	1	2	2	14'-0"
F11	Forex 2.0E-3000Fb LVL	1.75	9.5	1	2	2	10'-0"
F9	Forex 2.0E-3000Fb LVL	1.75	9.5			1	4'-0"

Joist (Flush)

Label	Description	Width	Depth	Qty	Piles	Pcs	Length
J8	NJH60U	3.5	9.5			29	19'-0"
J3	NJH	2.5	9.5			18	14'-0"
J2	NJH	2.5	9.5			9	12'-0"
J1	NJH	2.5	9.5			1	4'-0"

Rim Board

Label	Description	Width	Depth	Qty	Piles	Pcs	Length
R1	Norbord Rimboard Plus 1.125 X 9.5	1.125	9.5			14	12

Blocking

Label	Description	Width	Depth	Qty	Piles	Pcs	Length
BLK1	NJH	2.5	9.5			Varies	2'-0"

Hanger

Label	Pcs	Description	Skew	Slope	fasteners	Supported Member
H2	1	HUS1.61/10			30 16d	10 16d
H3	1	HUCQ1.61/9-SDS				
H5	8	LT259			4 10dx1 1/2	2 10dx1 1/2
H7	1	LT259				

NOTES:

1. Frame to verify dimensions on the architectural drawings.
2. Double joist only requires filler/backer ply when supporting another member using a face-mounted hanger.
3. Install 2x4 blocking @ 24" o/c under parallel non-load bearing walls.
4. Install single ply flush window header along inside face of rimboard/rimjoist.
5. Refer to Nascor specifier guide for installation works.
6. Squash blocks recommended to be installed at end bearing on all first level joists which support loading from above exceeding two levels floor or roof.
7. Load transfer blocks to be installed under at point loads.
8. It shall be the framer's responsibility that floor joists and beams are fastened as per the hanger manufacturer's standards.

Refer to Multiple Member Connection Detail to ply to ply nailing or bolting requirements.

Rim parallel to joists: 1-1/8" rimboard with 2"x4" block (1/16" longer than rim depth @ 16" o/c). All other components and structural elements supporting the floor system such as beams, walls, columns, and foundation walls and footings including anchorage of components and bracing for lateral stability are the responsibility of Others.

Hatch area represents ceramic tiled floor with an additional dead load of 5 PSF

The framing shown on this layout may deviate from the architectural and structural drawings. Project Engineer to review and approve the deviation prior to construction.

ARCHITECTURAL DRAWINGS:

JARDIN DESIGN GROUP INC.
64 Jardin Dr., Suite 3A, Vaughan, ON
Date: Rev 3; Aug 30, 2018
Project No: 18-24
Model: Clover 11A



Layout Name	CLOVER 11A-1
Design Method	LSD
Description	MINNISALE HOMES BRAMPTON, ONT.
Created	July 03, 2018
Builder	GREENPARK
Sales Rep	RM
Designer	RCO
Shipping	
Project	
Builder's Project	Kott Lumber Company
	14 Anderson Blvd Stouffville, Ontario Canada L4A 7X4 905-642-4400
Job Path	D:\Users\vochavilko\WORK FROM HOME\GREENPARK\MINNISALE HOMES\CLOVER 11A\FLOOR\REV CLOVER 11A.rvt
Second Floor	
Design Method	LSD
Building Code	NBCC 2010 / OBC 2012
Floor	
Live	40
Dead	15
Deflection Joist	
LL Span 1/	480
TL Span 1/	360
LL Cant 2L/	480
TL Cant 2L/	360
Deflection Girder	
LL Span 1/	360
TL Span 1/	240
LL Cant 2L/	480
TL Cant 2L/	360
Decking	
Deck	SPF Plywood
Thickness	5/8"
Fastener	Nailed & Glued
Vibration	
Ceiling	Gypsum 1/2"



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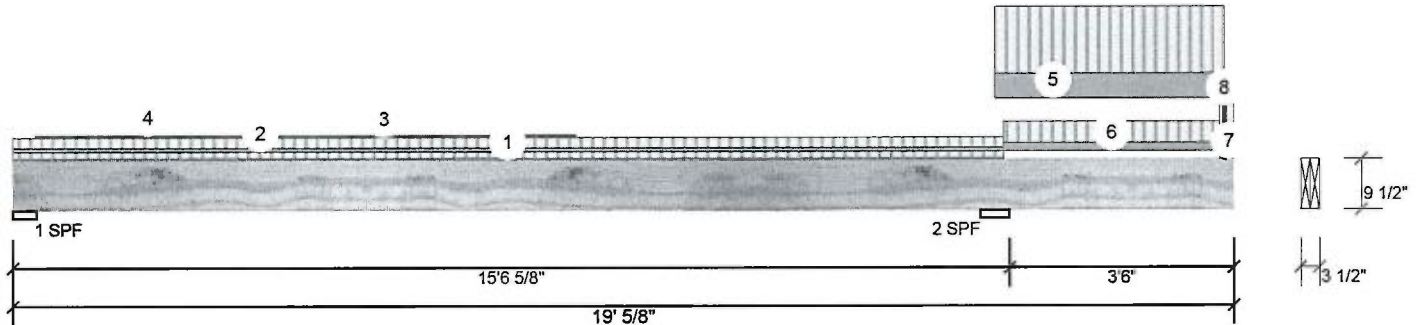
Client: GREENPARK
 Project:
 Address:

Date: 9/7/2018
 Designer: RCO
 Job Name: CLOVER 11A-1
 Project #:

Page 1 of 2

F10-A Forex 2.0E-3000Fb LVL 1.750" X 9.500" 2-Ply - PASSED

Level: Second Floor



Member Information

Unfactored Reactions UNPATTERNED lb (Uplift)

Type:	Girder	Application:	Floor (Residential)
Plies:	2	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		

Brg	Live	Dead	Snow	Wind
1	83	105	0	0
2	749	382	0	0

Bearings and Factored Reactions

Bearing	Length	Cap. React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	4.375"	4%	131 / 234	365 (-7) L	1.25D+1.5L
2 - SPF	5.500"	14%	478 / 1124	1602 LL	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Neg Moment	-2241 ft-lb	15'6 5/8"	22724 ft-lb	0.099 (10%)	1.25D+1.5L	L
Unbraced	-2241 ft-lb	15'6 5/8"	18268 ft-lb	0.123 (12%)	1.25D+1.5L	L
Pos Moment	1168 ft-lb	6'11 9/16"	20906 ft-lb	0.056 (6%)	1.25D+1.5L	L
Unbraced	1168 ft-lb	6'11 9/16"	18123 ft-lb	0.064 (6%)	1.25D+1.5L	L
Shear	855 lb	16'4 1/8"	9277 lb	0.092 (9%)	1.25D+1.5L	L
Perm Defl in.	0.018 (L/9997)	6'7 11/16"	0.501 (L/360)	0.040 (4%)	D	Uniform
LL Defl inch	0.055 (L/3278)	8'11 3/4"	0.501 (L/360)	0.110 (11%)	L	L
TL Defl inch	0.065 (L/2759)	7'5 1/8"	0.751 (L/240)	0.090 (9%)	D+L	L
LL Cant	0.084 (2L/995)	Rt Cant	0.200 (2L/480)	0.422 (42%)	L	L
TL Cant	0.092 (2L/911)	Rt Cant	0.300 (2L/360)	0.307 (31%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Tie-down connection required at bearing 1 for uplift 7 lb (Combination 0.9D+1.5L, Load Case L).
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on full section width.



September 13, 2018

Notes

Calculated Structural Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

1. Dry service conditions, unless noted otherwise
2. LVL not to be treated with fire retardant or corrosive chemicals

chemicals

Handling & Installation

1. LVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used
4. Design assumes top edge is laterally restrained
5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex
 APA: PR-L318

Kott Lumber Company
 14 Anderson Blvd, Ontario
 Canada
 L4A 7X4
 905-642-4400

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

This design

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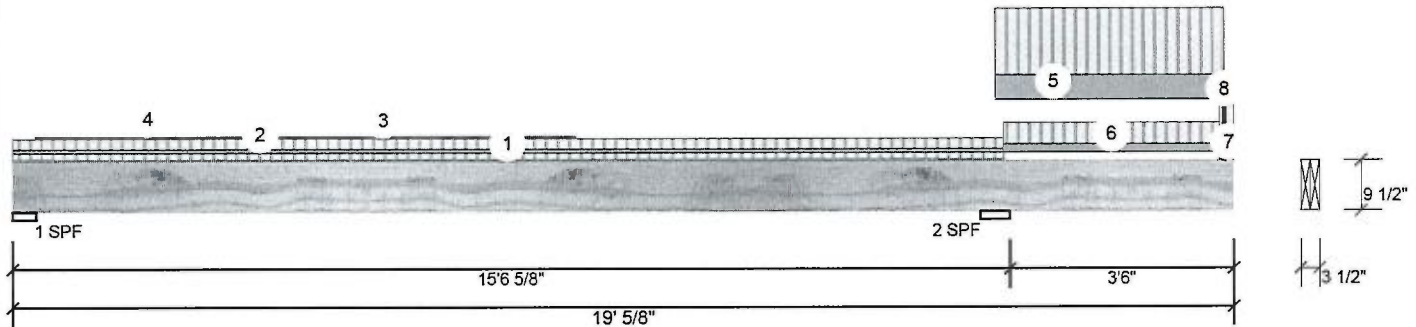
Client: GREENPARK
 Project:
 Address:

Date: 9/7/2018
 Designer: RCO
 Job Name: CLOVER 11A-1
 Project #:

Page 2 of 2

F10-A Forex 2.0E-3000Fb LVL 1.750" X 9.500" 2-Ply - PASSED

Level: Second Floor



ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 15-5-8	(Span)0-4-9	Top	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 15-5-6	(Span)0-7-7	Top	15 PSF	40 PSF	0 PSF	0 PSF	
3	Part. Uniform	0-4-5 to 8-9-7		Top	2 PLF	0 PLF	0 PLF	0 PLF	
4	Part. Uniform	0-4-5 to 8-9-8		Top	1 PLF	0 PLF	0 PLF	0 PLF	
5	Part. Uniform	15-3-14 to 18-10-14		Top	30 PLF	80 PLF	0 PLF	0 PLF	
6	Tie-In	15-5-6 to 18-10-0	(Span)1-3-7	Top	15 PSF	40 PSF	0 PSF	0 PSF	
7	Tie-In	18-10-0 to 19-0-10	(Span)2-8-0	Top	15 PSF	40 PSF	0 PSF	0 PSF	
8	Point	18-10-14		Near Face	56 lb	138 lb	0 lb	0 lb	F9
	Self Weight				8 PLF				

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

Calculated Structural Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

1. Dry service conditions, unless noted otherwise
2. LVL not to be treated with fire retardant or corrosive

chemicals**Handling & Installation**

1. LVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used
4. Design assumes top edge is laterally restrained
5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex
 APA: PR-L318

Kott Lumber Company
 14 Anderson Blvd, Ontario
 Canada
 L4A 7X4
 905-642-4400

NASCOR


This design is valid until 7/10/2021



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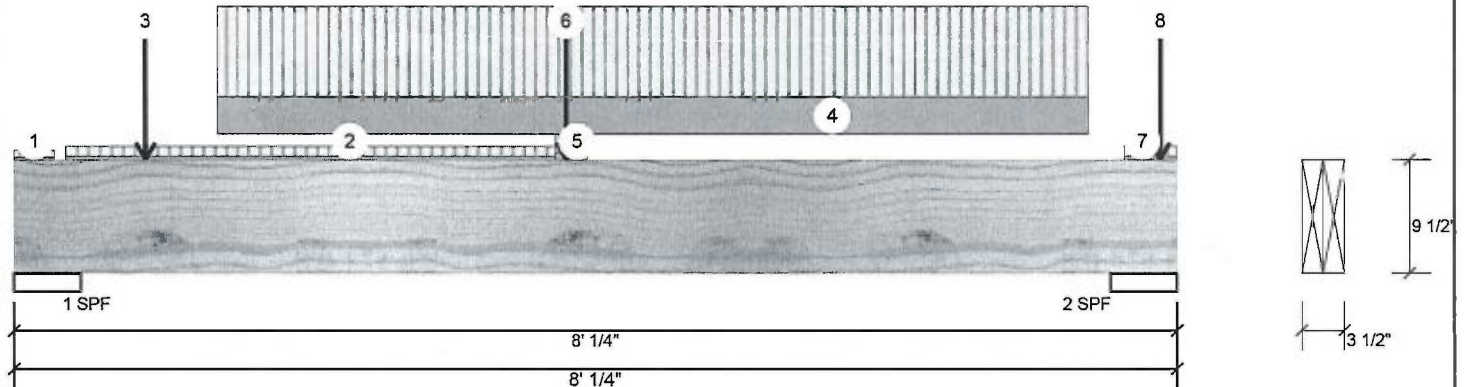
Client: GREENPARK
 Project:
 Address:

Date: 9/7/2018
 Designer: RCO
 Job Name: CLOVER 11A-1
 Project #:

Page 1 of 2

F11-A Forex 2.0E-3000Fb LVL 1.750" X 9.500" 2-Ply - PASSED

Level: Second Floor


Member Information
Unfactored Reactions UNPATTERNED lb (Uplift)

Type:	Girder	Application:	Floor (Residential)
Ply:	2	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		

Brg	Live	Dead	Snow	Wind
1	1033	462	0	0
2	1050	472	0	0

Bearings and Factored Reactions

Bearing	Length	Cap. React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	5.500"	18%	578 / 1550	2127 L	1.25D+1.5L
2 - SPF	5.500"	18%	590 / 1576	2166 L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	4121 ft-lb	3'9 3/4"	22724 ft-lb	0.181 (18%)	1.25D+1.5L	L
Unbraced	4121 ft-lb	3'9 3/4"	21692 ft-lb	0.190 (19%)	1.25D+1.5L	L
Shear	2360 lb	1'2 1/4"	9277 lb	0.254 (25%)	1.25D+1.5L	L
Perm Defl in.	0.019 (L/4469)	3'11 9/16"	0.241 (L/360)	0.080 (8%)	D	Uniform
LL Defl inch	0.044 (L/1986)	3'11 1/2"	0.241 (L/360)	0.180 (18%)	L	L
TL Defl inch	0.063 (L/1375)	3'11 1/2"	0.361 (L/240)	0.170 (17%)	D+L	L

Design Notes

- Girders are designed to be supported on the bottom edge only.
- Multiple plies must be fastened together as per manufacturer's details.
- Top loads must be supported equally by all plies.
- Top braced at bearings.
- Bottom braced at bearings.
- Lateral slenderness ratio based on full section width.



September 13, 2018

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 0-3-6	(Span)0-11-7	Top	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-4-4 to 3-8-14	(Span)1-4-9	Top	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	0-10-14		Near Face	86 lb	204 lb	0 lb	0 lb	J3
4	Part. Uniform	1-4-14 to 7-4-14		Near Face	109 PLF	258 PLF	0 PLF	0 PLF	
5	Tie-In	3-8-14 to 3-11-8	(Span)2-8-0	Top	15 PSF	40 PSF	0 PSF	0 PSF	
6	Point	3-9-12		Far Face	55 lb	134 lb	0 lb	0 lb	F9

Continued on page 2...

Notes

Calculated Structural Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

- Dry service conditions, unless noted otherwise
- LVL not to be treated with fire retardant or corrosive chemicals

Handling & Installation

- LVL beams must not be cut or drilled
- Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained
- Provide lateral support at bearing points to avoid lateral displacement and rotation

- For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex
 APA: PR-L318

Kott Lumber Company
 14 Anderson Blvd, Ontario
 Canada
 L4A 7X4
 905-642-4400

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

This design



isDesign™

Client: GREENPARK

Project:

Address:

Date: 9/7/2018

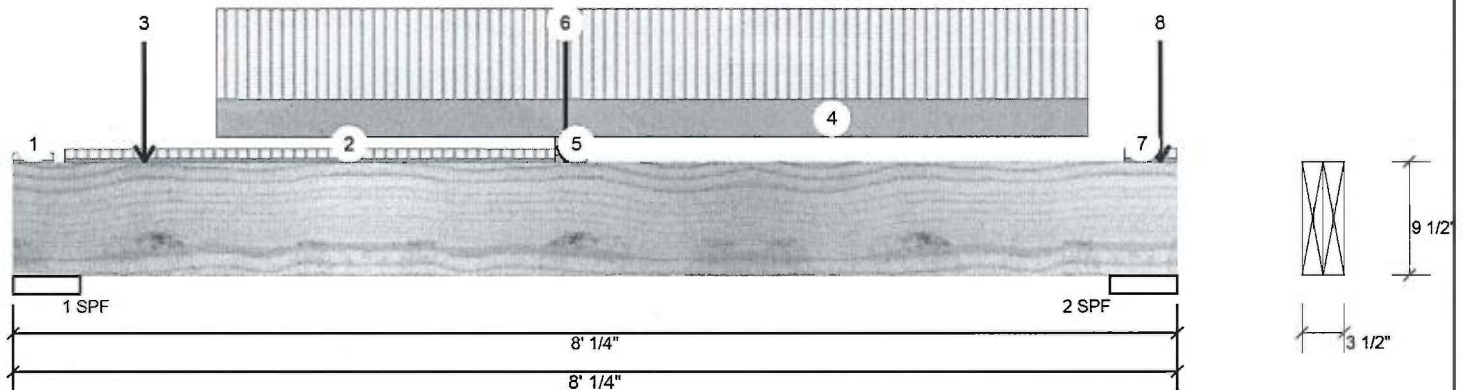
Designer: RCO

Job Name: CLOVER 11A-1

Project #:

Page 2 of 2

F11-A Forex 2.0E-3000Fb LVL 1.750" X 9.500" 2-Ply - PASSED Level: Second Floor



...Continued from page 1

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
7	Tie-In	7-7-14 to 8-0-4	(Span)1-4-9	Top	15 PSF	40 PSF	0 PSF	0 PSF	
8	Point	7-10-14		Near Face	33 lb	77 lb	0 lb	0 lb	J3
	Self Weight				8 PLF				

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

1. Dry service conditions, unless noted otherwise
2. LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

1. LVL beams must not be cut or drilled
2. Refer to manufacture's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used
4. Design assumes top edge is laterally restrained
5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex
APA: PR-L318

Kott Lumber Company
14 Anderson Blvd, Ontario
Canada
L4A 7X4
905-642-4400

NASCOR



This design is valid until 7/10/2021



isDesign™

Client: GREENPARK

Project:

Address:

Date: 9/7/2018

Designer: RCO

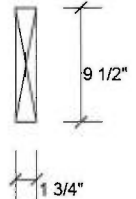
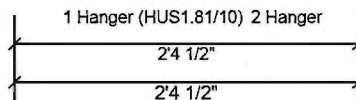
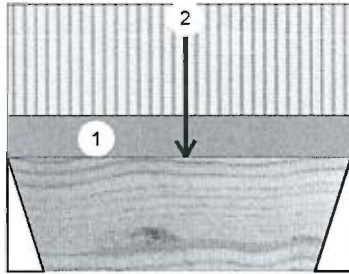
Job Name: CLOVER 11A-1

Project #:

Page 1 of 1

F9-A Forex 2.0E-3000Fb LVL 1.750" X 9.500" - PASSED

Level: Second Floor

**Member Information**

Type:	Girder	Application:	Floor (Residential)
Piles:	1	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	134	55	0	0
2	138	56	0	0

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - Hanger	3.000"	7%	68 / 201	269	L	1.25D+1.5L
2 - Hanger	3.000"	7%	70 / 207	278	L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	167 ft-lb	1'2 13/16"	11362 ft-lb	0.015 (1%)	1.25D+1.5L	L
Unbraced	167 ft-lb	1'2 13/16"	10730 ft-lb	0.016 (2%)	1.25D+1.5L	L
Shear	127 lb	1'4 3/4"	4638 lb	0.027 (3%)	1.25D+1.5L	L
Perm Defl in.	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
LL Defl inch	0.001 (L/30185)	1'2 13/16"	0.067 (L/360)	0.010 (1%)	L	L
TL Defl inch	0.001 (L/21630)	1'2 13/16"	0.100 (L/240)	0.010 (1%)	D+L	L

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.
- 4 Bottom braced at bearings.



September 13, 2018

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 2-4-8	(Span)3-9-9	Top	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	1-2-13		Far Face	34 lb	92 lb	0 lb	0 lb	J1
	Self Weight				4 PLF				

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

Calculated Structural Design is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

1. Dry service conditions, unless noted otherwise
2. LVL not to be treated with fire retardant or corrosive

chemicals**Handling & Installation**

1. LVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used
4. Design assumes top edge is laterally restrained
5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex
APA: PR-L318

Kott Lumber Company
14 Anderson Blvd, Ontario
Canada
L4A 7X4
905-642-4400

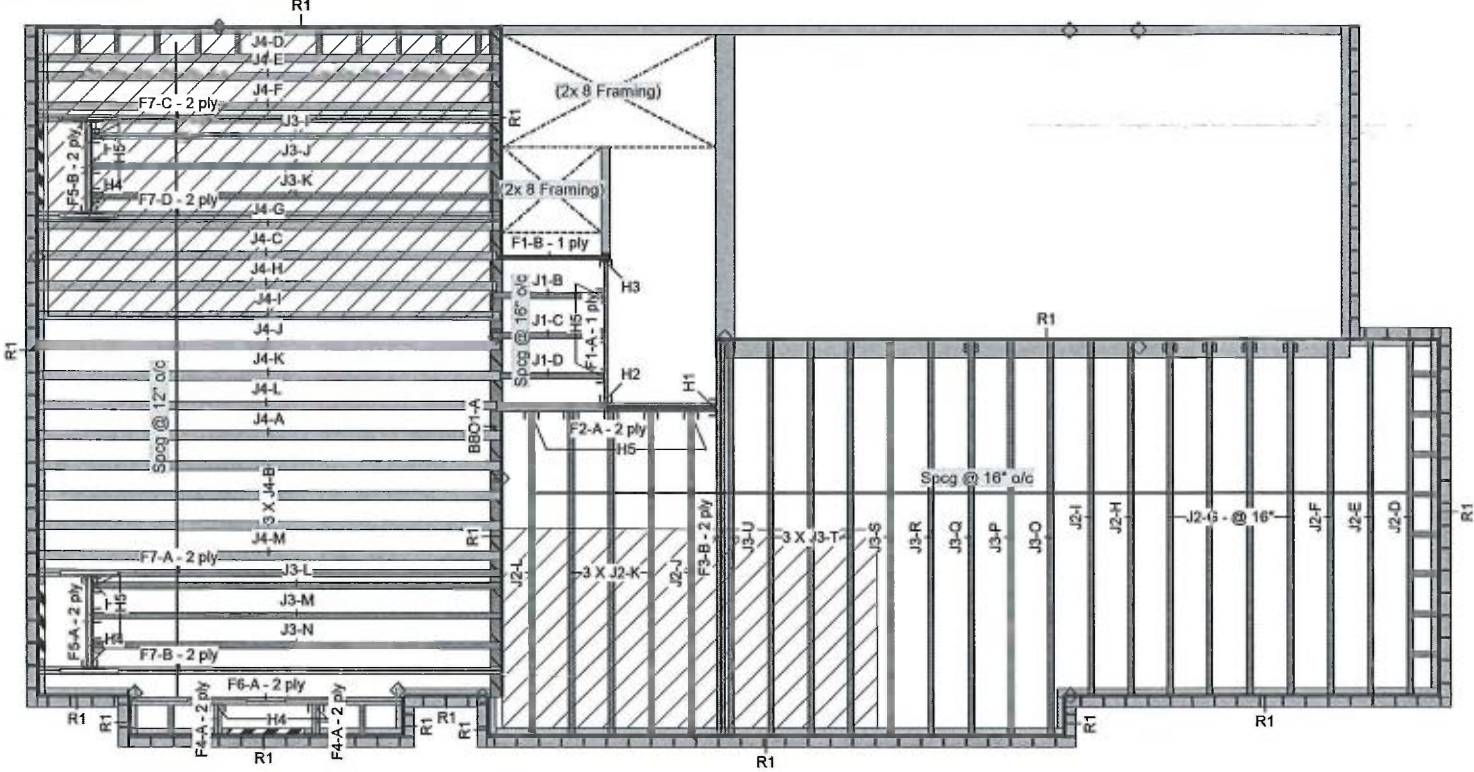
READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

This

NASCOR



Ground Floor

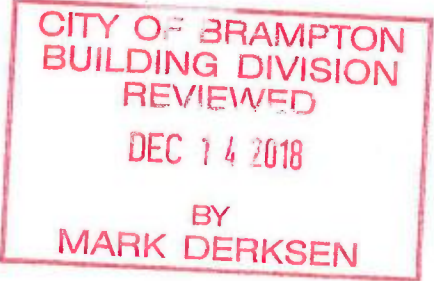


This certification is to confirm that:
1. The loads used in the calculation of the attached approved components conform to the floor assembly shown on this layout.
2. The floor joists comply with the Nascor span table for the loads and spacing shown on this layout.

The floor system must be assembled in accordance to the Nascor Specifier Guide. Multi-ply members must be attached together as per the included multiple member connection detail.
All other components and structural elements supporting the floor system such as beams, walls, columns and foundation walls and footings including anchorage of components and bracing for lateral stability are the responsibility of others.



September 13, 2018



Legend

	Load from Above
	Wall
	Wall Opening
	Norbord Rimboard Plus 1.125 X 9.5
	NJ 9.5
	NJ40U 9.5
	NJH 9.5
	Forex 2.0E-3000Fb LVL 1.75 X 9.5

1. OBC 2012 O.Reg 332/12 as amended
2. Nascor CCMC - 13535-R
3. LVL CCMC -14056-R
4. CAN/CSA-O86-09
5. CCMC -12787-R APA PR-L310(C)

All work shall conform to the Ontario Building Code O. Reg. 332/12 as amended

Engineered floor joists shall be installed in accordance with the supplier's layout and specifications forming part of the permit drawings.



Ground Floor LVL/LSL (Flush)

Label	Description	Width	Depth	Qty	Plies	Pcs	Length
F3	Forex 2.0E-3000Fb LVL	1.75	9.5	1	2	2	14-0-0
F2	Forex 2.0E-3000Fb LVL	1.75	9.5	1	2	2	8-0-0
F1	Forex 2.0E-3000Fb LVL	1.75	9.5			2	6-0-0

I Joist (Flush)

Label	Description	Width	Depth	Qty	Plies	Pcs	Length
F7	NJ	1.5	9.5	4	2	8	16-0-0
F6	NJ	1.5	9.5	1	2	2	10-0-0
F5	NJ	1.5	9.5	2	2	4	4-0-0
F4	NJ	1.5	9.5	2	2	4	2-0-0
J4	NJ40U	3.5	9.5			15	16-0-0
J3	NJH	2.5	9.5			15	14-0-0
J2	NJH	2.5	9.5			14	12-0-0
J1	NJH	2.5	9.5			3	4-0-0

Rim Board

Label	Description	Width	Depth	Qty	Plies	Pcs	Length
R1	Norbord Rimboard Plus 1.125 X 9.5	1.125	9.5			12	12

Blocking

Label	Description	Width	Depth	Qty	Plies	Pcs	Length
BLK1	NJH	2.5	9.5	LinFt		Varies	18-0-0

Hanger

Label	Pcs	Description	Skew	Slope	fasteners	Supported Member
H1	1	HGUS410			46 16d	16 16d
H2	1	HUS1.81/10			30 16d	10 16d
H3	1	HUCQ1.81/9-SDS				
H4	6	LT2-159			4 10dx1 1/2	2 10dx1 1/2
H5	14	LT259			4 10dx1 1/2	2 10dx1 1/2

NOTES:

1. Framer to verify dimensions on the architectural drawings.
2. Double joist only require filler/backer ply when supporting another member using a face-mounted hanger.
3. Install 2x4 blocking @ 24" o/c under parallel non-load bearing walls.
4. Install single-ply flush window header along inside face of rimboard/rimjoist.
5. Refer to Nascor specifier guide for installation works.
6. Squash blocks recommended to be installed at end bearing on all first level joists which support loading from above exceeding two levels floor or roof.
7. Load transfer blocks to be installed under all point loads.
8. It shall be the framer's responsibility that floor joists and beams are fastened as per the hanger manufacturer's standards.

Refer to Multiple Member Connection Detail to ply to ply nailing or bolting requirements.

Rim parallel to joists: 1-1/8" rimboard with 2"x 4" block (1/16" longer than rim depth @ 16" o/c). All other components and structural elements supporting the floor system such as beams, walls, columns, and foundation walls and footings including anchorage of components and bracing for lateral stability are the responsibility of Others.

Hatch area represents ceramic tiled floor with an additional dead load of 5 PSF

The framing shown on this layout may deviate from the architectural and structural drawings. Project Engineer to review and approve the deviation prior to construction.

ARCHITECTURAL DRAWINGS:

JARDIN DESIGN GROUP INC.
64 Jardin Dr., Suite 3A, Vaughan, ON
Date: Rev.3; Aug.30,2018
Project No: 18-24
Model: Clover 11A

Layout Name
CLOVER 11A-1

Design Method
LSD

Description
MINNISALE HOMES
BRAMPTON, ONT.

Created
July 03, 2018

Builder
GREENPARK

Sales Rep
RM

Designer
RCO

Shipping

Project

Builder's Project

Kott Lumber Company
14 Anderson Blvd
Stouffville, Ontario
Canada
L4A 7X4
905-642-4400

Job Path
D:\Users\rochavillo\WORK FROM HOME\GREENPARK\MINNISALE HOMES\CLOVER 11A\FLOOR\REV\ CLOVER 11A.rvt

Ground Floor

Design Method
LSD

Building Code
NBCC 2010 / OBC 2012

Floor

Loads

Live
40

Dead
15

Deflection Joist

LL Span L/
480

TL Span L/
360

LL Cant 2L/
480

TL Cant 2L/
360

Deflection Girder

LL Span L/
360

TL Span L/
240

LL Cant 2L/
480

TL Cant 2L/
360

Decking

Deck
SPF Plywood

Thickness
3/4"

Fastener
Nailed & Glued

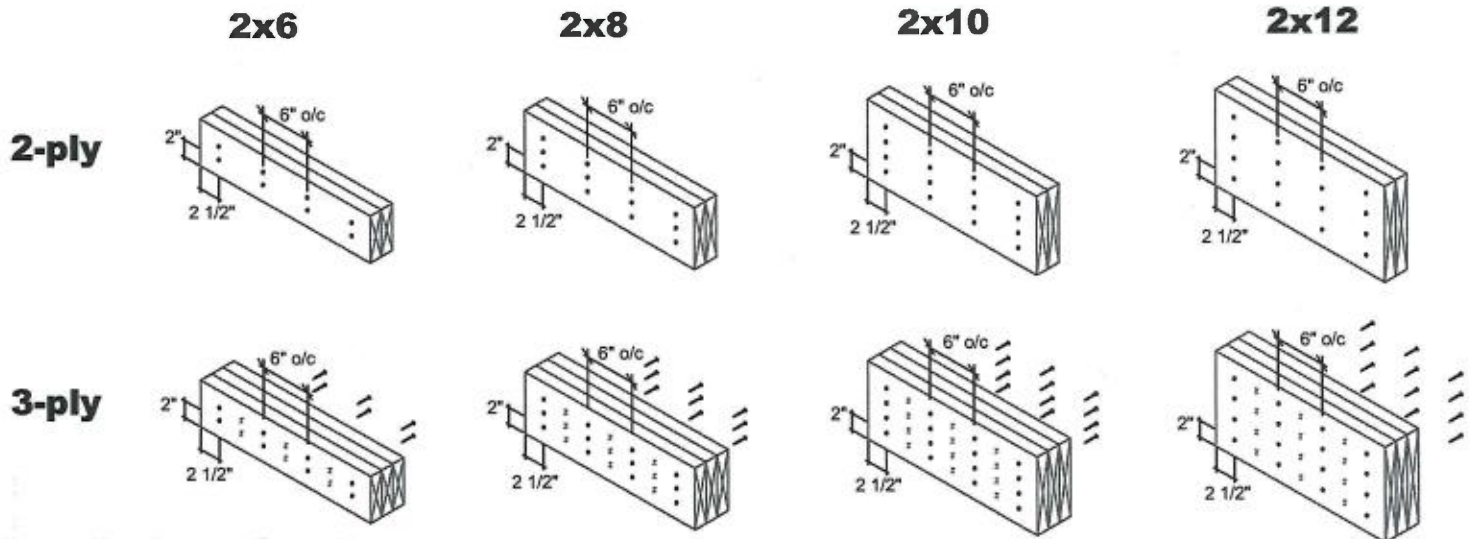
Vibration

[illegible]

- | | |
|-----------|----------------|
| Decking | |
| Deck | SPF Plywood |
| Thickness | 5/8" |
| Fastener | Nailed & Glued |
| Vibration | |
| Ceiling: | Gypsum 1/2" |

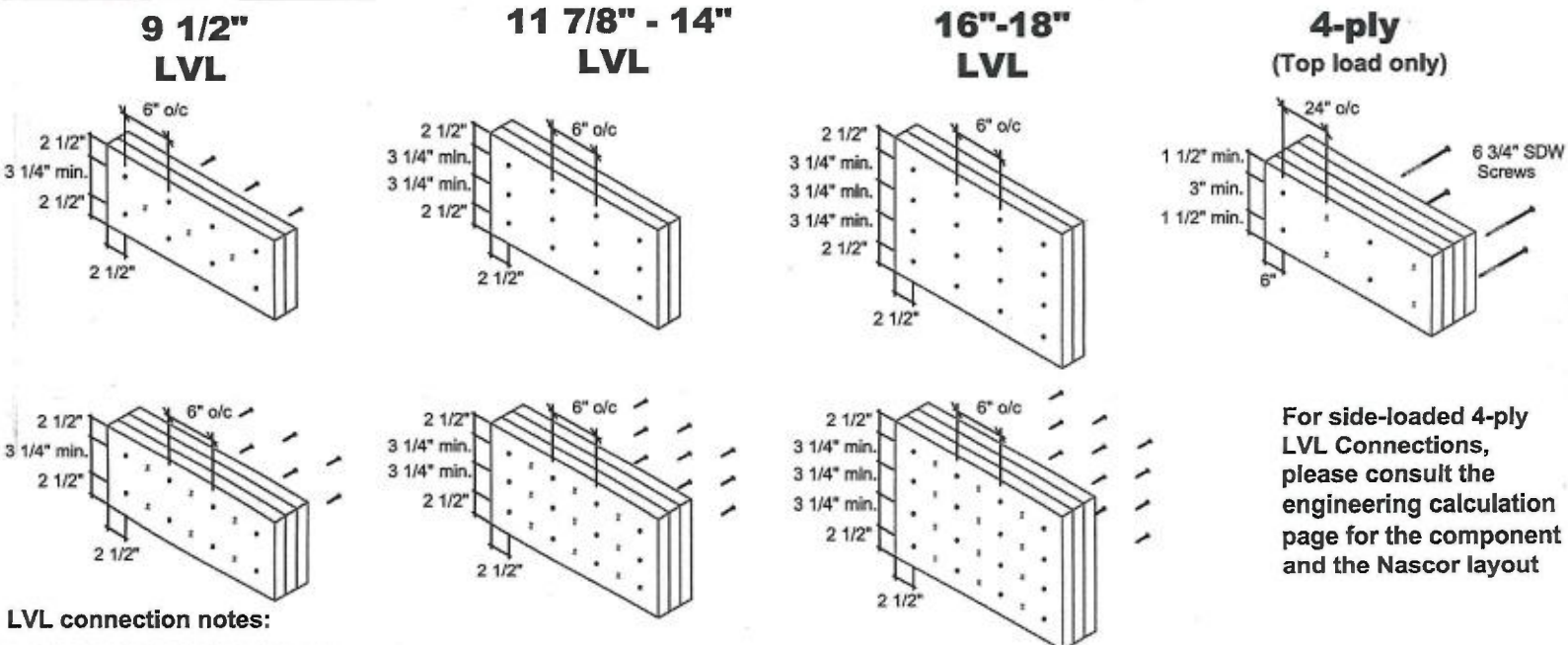
MULTIPLE MEMBER CONNECTIONS

Conventional Connections (for uniform distributed loads)



Conventional connection notes:
-Nails to be 3" 10d spiral wire nails.
-Nails to be located a minimum of 2" from the top and bottom of the member. Start all nails a minimum of 2 1/2" in from ends.
-Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail driven from the opposite side.

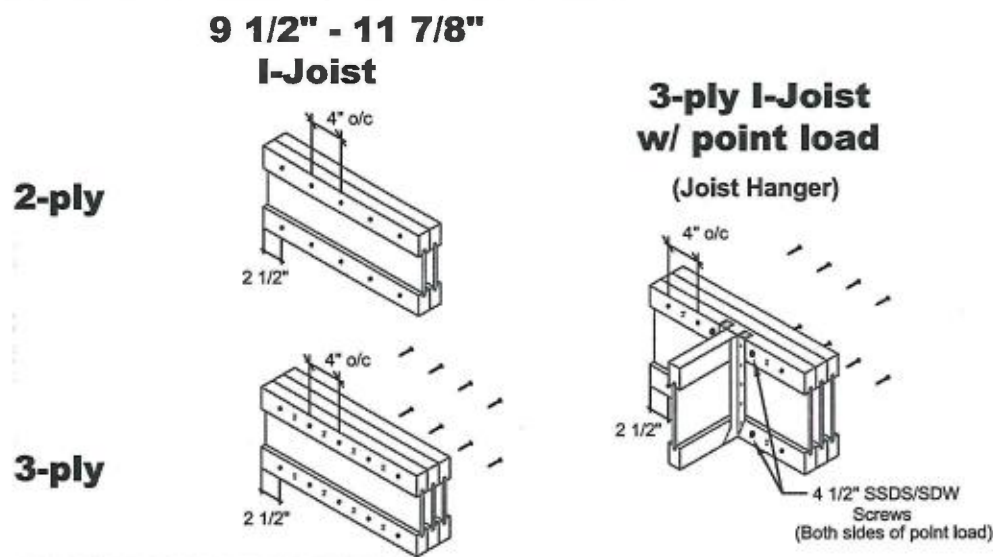
LVL Connections (for uniform distributed loads)



LVL connection notes:
-Nails to be 3 1/2" spiral wire nails.
-Nails to be located a minimum of 2 1/2" from the top and bottom of the member. Start all nails a minimum of 2 1/2" in from ends.
-Minimum 3 1/4" spacing between rows.
-Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail or screw driven from the opposite side.

For side-loaded 4-ply LVL Connections, please consult the engineering calculation page for the component and the Nascor layout

Vertical I-Joist Connections (for uniform distributed loads)



Vertical I-Joist connection notes:
-Nails to be 3" spiral wire nails.
-Nails to be located at centre of top and bottom flanges. Start all nails a minimum of 2 1/2" in from ends.
-Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail driven from the opposite side.