

ASPHALT SHINGLES
12" FINISH O.H.
R.T.M.C
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:

HGUS26-2 -(XX)
LJS26DS -(V)
LUS24 -(O)

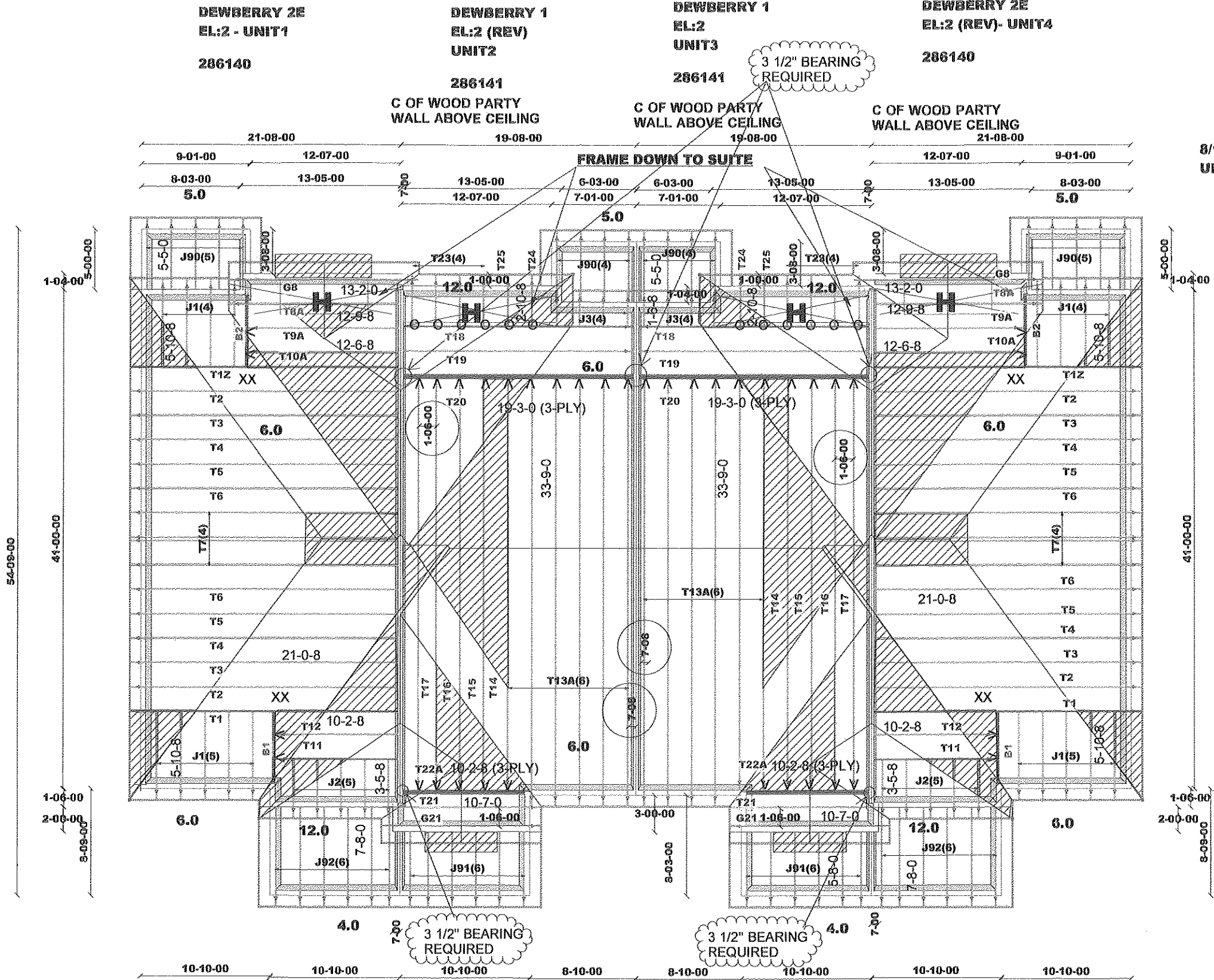
DENOTES:

H -16" RAISED PLATE

ALL B- 2-2X10 BEAMS (FLUSH)

T-170534

DENOTES:
CONVENTIONAL
FRAMING



8/12 PITCHES (TYP.)
UNLESS NOTED

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART 9 OF THE OBC. LATEST EDITION
ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"x4"SPF@24"o.c.
WITH A 2"x4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.
POSTS LONGER THAN 6' TO BE Laterally Braced so that the distance
BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN LOADS:

SNOW LOAD 25.6 PSF
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF



Job Track: 43954
Layout ID: 286139
Plan Log: 93671

Builder / Location:
GREENPARK HOMES / WATERDOWN
Project: **RUSSEL GAREDENS PH.2**
Date: 8/11/2017 Designer: sonny

Model / Elevation:
BLOCK214/ UNIT1TO4

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TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.



Delivery Shiplist

DATE	09/11/17
SALES REP	Mario

JOB TRACK:43954	LAYOUT ID: 286140	LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH2	SUB-BUILDER:	
MODEL: DEWBERRY 2E	ELEVATION: 2- UNIT1 OR 2(REV)-UNIT4	

ROOF TRUSSES

ROOF TRUSS SPACING:24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	T1 HALF HIP	8.00 0.00	21-00-08	04-01-04	2 X 6	2 X 6	01-03-08 00-00-00	01-04-13 04-01-04	121.25 75.34		
	1	T1Z HALF HIP	8.00 0.00	21-00-08	04-01-04	2 X 6	2 X 6	01-03-08 00-00-00	01-04-13 04-01-04	121.25 75.34		
	2	T2 HALF HIP	8.00 0.00	21-00-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-01-04	178.24 112.66		
	2	T3 HALF HIP	8.00 0.00	21-00-08	06-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 06-01-04	179.12 112.00		
	2	T4 HALF HIP	8.00 0.00	21-00-08	07-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 07-01-04	188.76 118.68		
	2	T5 HALF HIP	8.00 0.00	21-00-08	08-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 08-01-04	203.36 127.66		
	2	T6 HALF HIP	8.00 0.00	21-00-08	09-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 09-01-04	214.60 134.34		
	4	T7 HALF HIP	8.00 0.00	21-00-08	10-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 10-01-04	448.48 280.00		
	1	T8A COMMON	8.00 0.00	12-09-08	05-09-08	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-07-13	54.73 35.17		
	1	G8 COMMON	8.00 0.00	13-02-00	05-09-08	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	55.88 36.00		
	1	T9A HALF HIP	8.00 0.00	12-06-08	06-02-08	2 X 4	2 X 4	00-00-00 00-00-00	02-10-13 06-02-08	58.88 37.67		
	1	T10A HALF HIP	8.00 0.00	12-06-08	08-02-08	2 X 4	2 X 4	00-00-00 00-00-00	02-10-13 08-02-08	65.56 41.50		
	1	T11 HALF HIP	8.00 0.00	10-02-08	05-04-00	2 X 4	2 X 4	00-00-00 00-00-00	01-04-13 05-04-00	44.82 29.00		
	1	T12 HALF HIP	8.00 0.00	10-02-08	07-04-00	2 X 4	2 X 4	00-00-00 00-00-00	01-04-13 07-04-00	56.66 35.83		
	9	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	151.11 96.03		
	5	J2 JACK-OPEN	12.00 0.00	03-05-08	05-04-00	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 05-04-00	68.05 46.65		
	5	J90 JACK	5.00 0.00	05-05-00	02-08-07	2 X 4	2 X 4	01-03-08 00-00-00	00-05-06 02-08-07	77.00 50.00		
	6	J92 JACK	4.00 0.00	07-08-00	02-10-10	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-10-10	136.62 87.00		

TOTAL # TRUSS= 47.00

TOTAL BFT OF ALL TRUSSES=

1530.87 BFT. TOTAL WEIGHT OF ALL TRUSSES= 2424.37 LBS.



Delivery Shiplist

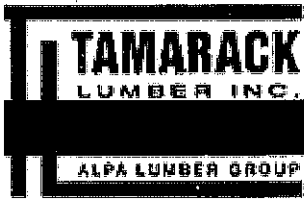
DATE	09/11/17
SALES REP	Mario

JOB TRACK: 43954	LAYOUT ID: 286140	LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH ²	SUB-BUILDER:	
MODEL: DEWBERRY 2E	ELEVATION: 2- UNIT1 OR 2(REV)-UNIT4	

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
4	Hangers	LJS26DS	

TOTAL # ITEMS= 6.00



Delivery Shiplist

DATE	09/11/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 286141 LOCATION:
 BUILDER: GREENPARK - RUSSELL GARDENS PH 2 SUB-BUILDER:
 MODEL: DEWBERRY 1 ELEVATION: 2 (REV)-UNIT2 OR 2-UNIT3

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	6	T13A COMMON	6.00 0.00	33-09-00	11-01-00	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-08	892.62 547.98		
	1	T14 HALF HIP	6.00 0.00	33-09-00	06-08-08	2 X 4	2 X 4	00-00-00 00-00-00	01-02-00 06-08-08	145.66 92.84		
	1	T15 HALF HIP	6.00 0.00	33-09-00	08-00-08	2 X 4	2 X 4	00-00-00 00-00-00	01-02-00 08-00-08	148.39 92.17		
	1	T16 ROOF	12.00 0.00	33-09-00	09-04-08	2 X 4	2 X 4	00-00-00 00-00-00	02-10-08 09-04-08	179.78 111.00		
	1	T17 ROOF	12.00 0.00	33-09-00	10-04-08	2 X 4	2 X 4	00-00-00 00-00-00	02-10-08 10-04-08	172.87 106.50		
	1	T18 ROOF	8.00 0.00	19-03-00	06-01-00	2 X 4	2 X 4	00-00-00 00-00-00	01-11-04 06-01-00	81.02 51.67		
	1	T19 ROOF	8.00 0.00	19-03-00	08-01-00	2 X 4	2 X 4	00-00-00 00-00-00	02-11-04 08-01-00	88.69 56.33		
	1 3 Ply	T20 ROOF	8.00 0.00	19-03-00	10-01-00	2 X 6	2 X 6	00-00-00 00-00-00	03-11-04 10-01-00	463.47 287.01		
	1	T21 COMMON	8.00 0.00	10-07-00	04-11-02	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	44.34 28.17		
	1	G21 GABLE	8.00 0.00	10-07-00	04-11-02	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	43.89 29.17		
	1 3 Ply	T22A COMMON	8.00 0.00	10-02-08	04-11-02	2 X 6	2 X 6	00-00-00 00-00-00	01-04-13 01-07-13	177.66 111.99		
	4	T23 MONO TRUSS	12.00 0.00	02-10-08	02-11-00	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 06-01-00	81.40 54.00		
	1	T24 HALF HIP	12.00 0.00	02-10-08	02-08-13	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 04-00-13	19.07 13.00		
	1	T25 HALF HIP	12.00 0.00	02-10-08	04-00-13	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 05-04-13	22.10 15.50		
	4	J3 JACK-OPEN	6.00 0.00	01-06-08	01-11-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 01-11-04	25.32 18.68		
	4	J90 JACK	5.00 0.00	05-05-00	02-08-07	2 X 4	2 X 4	01-03-08 00-00-00	00-05-06 02-08-07	61.60 40.00		
	6	J91 JACK	4.00 0.00	05-08-00	02-02-10	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-02-10	89.64 55.98		

TOTAL # TRUSS= 40.00

TOTAL BFT OF ALL TRUSSES=

1711.99 BFT. TOTAL WEIGHT OF ALL TRUSSES= 2737.52 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
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Delivery Shiplist

DATE	09/11/17
SALES REP	Mario

JOB TRACK: 43954	LAYOUT ID: 286141	LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH 2	SUB-BUILDER:	
MODEL: DEWBERRY 1	ELEVATION: 2 (REV)-UNIT2 OR 2-UNIT3	

HARDWARE

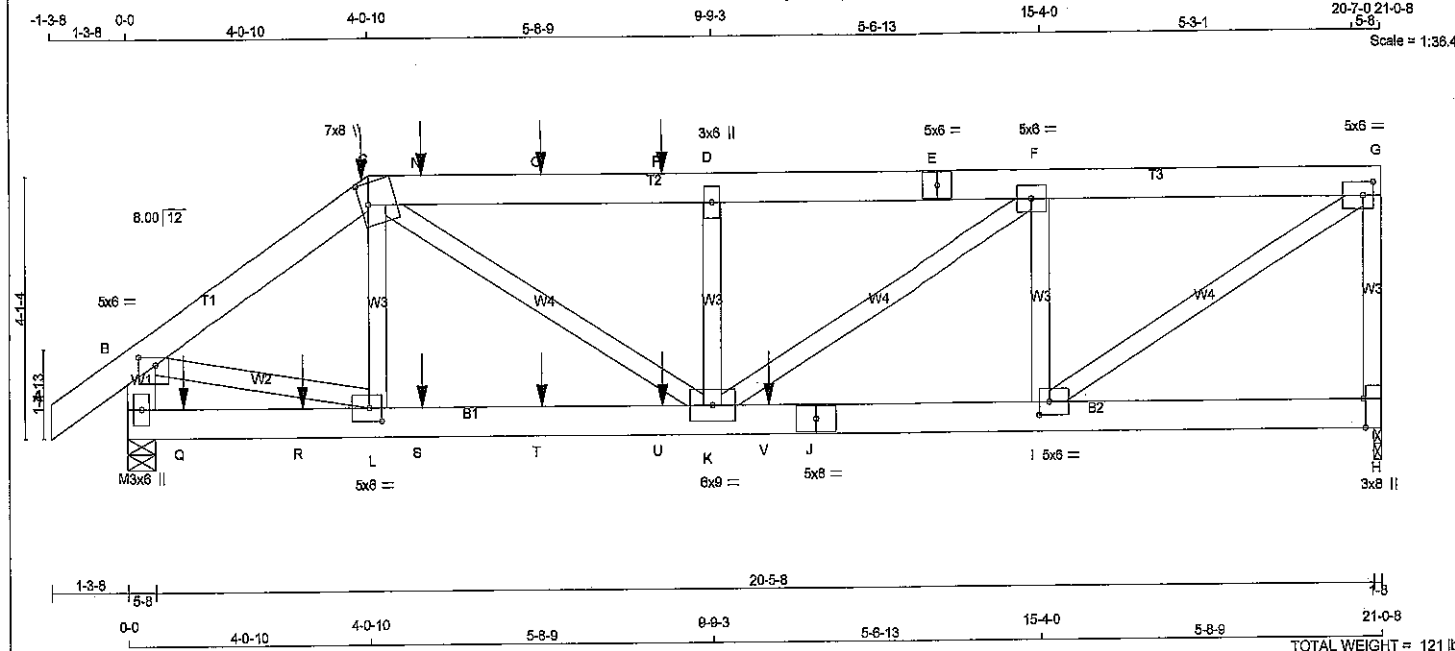
QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
15	Hangers	LJS26DS	
6	Hangers	LUS24	

TOTAL # ITEMS= 21.00

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
286140	T1	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER				N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x8	DRY	No.2	SPF			
C - E	2x8	DRY	No.2	SPF			
E - G	2x8	DRY	No.2	SPF			
H - G	2x4	DRY	No.2	SPF			
M - B	2x6	DRY	No.2	SPF			
M - J	2x8	DRY	2100F 1.8E	SPF			
J - H	2x8	DRY	2100F 1.8E	SPF			
ALL WEBS	2x4	DRY	No.2	SPF			
EXCEPT							
DRY: SEASONED LUMBER.							

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	X
B	TMVW-p	MT20	5.0	6.0	1.50 3.25
C	TTWW+m	MT20	7.0	8.0	4.00 1.50
D	TMW-w	MT20	3.0	6.0	
E	TS-t	MT20	5.0	6.0	
F	TMWW-t	MT20	5.0	6.0	
G	TMVW-t	MT20	5.0	6.0	2.50 2.00
H	BMV1+p	MT20	3.0	6.0	Edge 0.50
I	BMWW-t	MT20	5.0	6.0	2.50 2.00
J	BS-t	MT20	5.0	6.0	
K	BMWWH-t	MT20	6.0	9.0	
L	BMWW-t	MT20	5.0	6.0	2.50 2.50
M	BMV1+p	MT20	3.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 244.5 lbs FACTORED DOWN AT 4-0-11, 106.3 lbs FACTORED DOWN AT 4-11-4, AND 101.4 lbs FACTORED DOWN AT 6-11-4, AND 101.4 lbs FACTORED DOWN AT 8-11-4 ON TOP CHORD, AND 71.4 lbs FACTORED DOWN AT 11-4, 69.9 lbs FACTORED DOWN AT 2-11-4, 69.9 lbs FACTORED DOWN AT 4-11-4, 69.9 lbs FACTORED DOWN AT 6-11-4, AND 69.9 lbs FACTORED DOWN AT 8-11-4, AND 900.5 lbs FACTORED DOWN AT 10-8-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED		MAXIMUM FACTORED		INPUT	REQD
	GROSS	REACTION	GROSS	REACTION		
	VERT	HORZ	DOWN	HORZ	BRG	BRG
H	1835	0	1835	0	1-8	1-8
M	2302	0	2302	0	5-6	5-6

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
H	1494	849 / 0	327 / 0	0 / 0	0 / 0	318 / 0	0 / 0
M	1870	1069 / 0	405 / 0	0 / 0	0 / 0	396 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.05 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

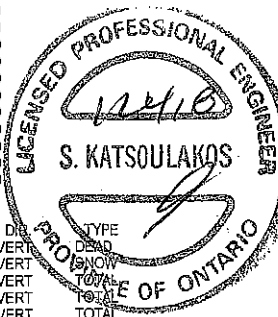
LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED	FACTORED	VERT. LOAD	MAX. UNBRACED	MEMB.	MAX. FACTORED	MAX. UNBRACED
	FORCE (LBS)	(PLF)	LC1 (LBS)	LENGTH		FORCE (LBS)	MAX. UNBRACED
FR-TO					FR-TO		
A-B	0 / 33	-84.3	-84.3 0.07 (1)	10.00	L-C	-257 / 215	0.04 (1)
B-C	-2612 / 0	-84.3	-84.3 0.17 (1)	4.95	C-K	0 / 1754	0.31 (1)
C-N	-3638 / 0	-84.3	-84.3 0.42 (1)	4.05	K-D	-880 / 0	0.12 (1)
N-O	-3638 / 0	-84.3	-84.3 0.42 (1)	4.05	K-F	0 / 1360	0.25 (1)
O-P	-3638 / 0	-84.3	-84.3 0.42 (1)	4.05	F-G	-1258 / 0	0.22 (1)
P-D	-3638 / 0	-84.3	-84.3 0.42 (1)	4.05	I-G	0 / 2936	0.52 (1)
D-E	-3638 / 0	-84.3	-84.3 0.31 (1)	4.19	B-L	0 / 2218	0.39 (1)
E-F	-3638 / 0	-84.3	-84.3 0.31 (1)	4.19			
F-G	-2475 / 0	-84.3	-84.3 0.23 (1)	5.00			
H-G	-1785 / 0	0.0	0.0 0.41 (1)	8.21			
M-B	-2210 / 0	0.0	0.0 0.18 (1)	8.84			
M-Q	0 / 0	-28.0	-28.0 0.07 (3)	10.00			
Q-R	0 / 0	-28.0	-28.0 0.07 (3)	10.00			
R-L	0 / 0	-28.0	-28.0 0.07 (3)	10.00			
L-S	0 / 2157	-28.0	-28.0 0.22 (1)	10.00			
S-T	0 / 2157	-28.0	-28.0 0.22 (1)	10.00			
T-U	0 / 2157	-28.0	-28.0 0.22 (1)	10.00			
U-K	0 / 2157	-28.0	-28.0 0.22 (1)	10.00			
K-V	0 / 2475	-28.0	-28.0 0.33 (1)	10.00			
V-J	0 / 2475	-28.0	-28.0 0.33 (1)	10.00			
J-I	0 / 2475	-28.0	-28.0 0.33 (1)	10.00			
I-H	0 / 0	-28.0	-28.0 0.08 (2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	4-0-11	-23	-25		FRONT	VERT	DEAD
C	4-0-11	-222	-222		FRONT	VERT	SNOW
N	4-11-4	-106	-106		FRONT	VERT	TOTAL
O	6-11-4	-101	-101		FRONT	VERT	TOTAL
P	8-11-4	-101	-101		FRONT	VERT	TOTAL
Q	11-4	-41	-71		FRONT	VERT	TOTAL
R	2-11-4	-40	-70		FRONT	VERT	TOTAL
S	4-11-4	-40	-70		FRONT	VERT	TOTAL
T	6-11-4	-40	-70		FRONT	VERT	TOTAL
U	8-11-4	-40	-70		FRONT	VERT	TOTAL
V	10-8-8	-901	-901		FRONT	VERT	TOTAL



DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.70")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.12")
ALLOWABLE DEFL.(TL) = $L/360$ (0.70")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.18")

CSI: TC=0.42/1.00 (C-D:1), BC=0.33/1.00 (I-K:1),
WB=0.52/1.00 (G-I:1), SS=0.56/1.00 (I-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	618	354	1657 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.73 (J) (INPUT = 1.00)

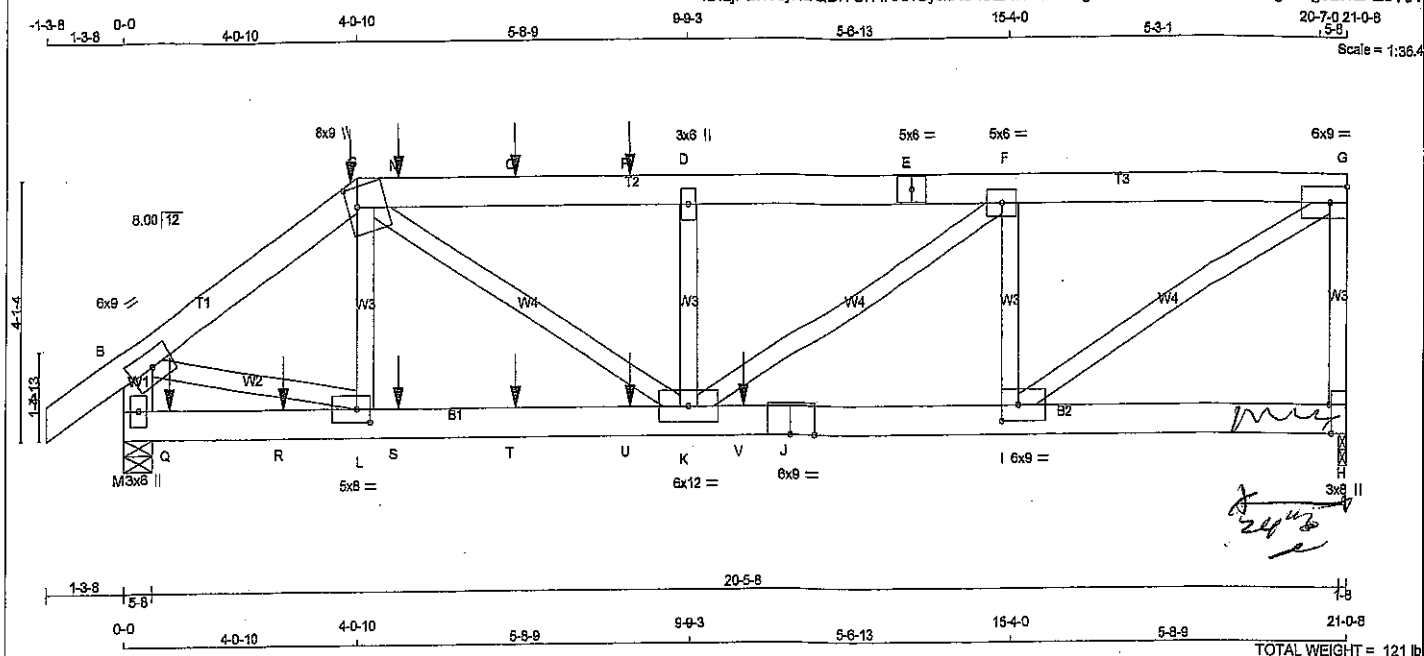
DRWG NO. TAM 5066-18
STRUCTURAL
COMPONENT ONLY

JOB NAME 286185	TRUSS NAME T1Z	QUANTITY 1	PLY 1	JOB DESC. TRUSS DESC.	43954	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID: LJPBf7XlyxdQBIYCH4AXRGyehXO-IJZX6EHNILMgfh0NVnQKMhU4mUmbglWgJEHEAsY0Y



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x6	DRY	No.2
C - E	2x6	DRY	No.2
E - G	2x6	DRY	No.2
H - G	2x4	DRY	No.2
M - B	2x6	DRY	No.2
M - J	2x6	DRY	2100F 1.8E
J - H	2x6	DRY	2100F 1.8E

ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
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DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	6.0	9.0	
C	TTWW+m	MT20	8.0	9.0	3.75 2.00
D	TMVW-w	MT20	3.0	6.0	
E	TS-t	MT20	5.0	6.0	
F	TMVW-t	MT20	5.0	6.0	
G	TMVW-t	MT20	6.0	9.0	Edge
H	BMV1+p	MT20	3.0	8.0	Edge 0.50
I	BMVW-t	MT20	6.0	9.0	3.00 3.50
J	BS-t	MT20	6.0	9.0	
K	BMVWW-t	MT20	6.0	12.0	
L	BMVW-t	MT20	5.0	8.0	2.50 2.75
M	BMV1+p	MT20	3.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 113.4 lbs FACTORED DOWN AT 4-9-4, 101.4 lbs FACTORED DOWN AT 6-9-4, AND 101.4 lbs FACTORED DOWN AT 8-9-4, AND 483.4 lbs FACTORED DOWN AT 4-0-10 ON TOP CHORD, AND 75.5 lbs FACTORED DOWN AT 9-4, 69.9 lbs FACTORED DOWN AT 2-9-4, 69.9 lbs FACTORED DOWN AT 4-9-4, 69.9 lbs FACTORED DOWN AT 6-9-4, AND 69.9 lbs FACTORED DOWN AT 8-9-4, AND 1147.2 lbs FACTORED DOWN AT 10-8-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

SCAB JOINT #4 WITH 2 X 6 #2
SPF 24" LONG, L SIDE(S) USING 2
ROWS(S) 3/4" ARDOX SPIRAL COMMON WIRE
NAILS AT 2" O/C STAGGERED (16 NAILS/SCAB).
REQUIRED FOR 1/2" BRG PROVISION.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	UPLIFT	IN-SX
H	3070	0	0	1-8
M	3444	0	0	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX/MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	
H	2492	1430 / 0	537 / 0	0 / 0	0 / 0	526 / 0	0 / 0	
M	2808	1591 / 0	618 / 0	0 / 0	0 / 0	599 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 33	-84.3 -84.3	0.07 (5)	10.00	L-C	-392 / 343	0.07 (1)
B-C	-4097 / 0	-84.3 -84.3	0.22 (1)	4.06	C-K	0 / 2692	0.48 (1)
C-N	-5858 / 0	-162.4 -162.4	0.74 (1)	2.86	K-D	-1135 / 0	0.19 (1)
N-O	-5858 / 0	-162.4 -162.4	0.74 (1)	2.86	D-F	0 / 1049	0.34 (1)
O-P	-5858 / 0	-162.4 -162.4	0.74 (1)	2.86	F-I	-2048 / 0	0.35 (1)
P-D	-5858 / 0	-162.4 -162.4	0.74 (1)	2.86	I-G	0 / 4778	0.84 (1)
D-E	-5858 / 0	-162.4 -162.4	0.80 (1)	3.11	G-L	0 / 3477	0.61 (1)
E-F	-5858 / 0	-162.4 -162.4	0.80 (1)	3.11			
F-G	-4028 / 0	-162.4 -162.4	0.47 (1)	3.81			
H-G	-2861 / 0	0.0 0.0	0.68 (1)	4.94			
M-B	-3302 / 0	0.0 0.0	0.23 (1)	5.79			
M-Q	0 / 0	-54.0 -54.0	0.11 (3)	10.00			
Q-R	0 / 0	-54.0 -54.0	0.11 (3)	10.00			
R-L	0 / 0	-54.0 -54.0	0.11 (3)	10.00			
L-S	0 / 3384	-54.0 -54.0	0.32 (1)	10.00			
S-T	0 / 3384	-54.0 -54.0	0.32 (1)	10.00			
T-U	0 / 3384	-54.0 -54.0	0.32 (1)	10.00			
U-K	0 / 3384	-54.0 -54.0	0.32 (1)	10.00			
K-V	0 / 4028	-54.0 -54.0	0.48 (1)	10.00			
V-J	0 / 4028	-54.0 -54.0	0.48 (1)	10.00			
J-I	0 / 4028	-54.0 -54.0	0.48 (1)	10.00			
I-H	0 / 0	-54.0 -54.0	0.14 (2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR	TYPE
C	4-0-10	-249	-249		FRONT	VERT	TOTAL
C	4-0-10	-23	-25		FRONT	VERT	DEAD
C	4-0-10	-222	-222		FRONT	VERT	SNOW
N	4-9-4	-113	-113		FRONT	VERT	TOTAL
O	8-9-4	-101	-101		FRONT	VERT	TOTAL
P	8-9-4	-101	-101		FRONT	VERT	TOTAL
Q	9-4	-43	-76		FRONT	VERT	TOTAL
R	2-9-4	-40	-70		FRONT	VERT	TOTAL
S	4-9-4	-40	-70		FRONT	VERT	TOTAL
T	6-9-4	-40	-70		FRONT	VERT	TOTAL
U	8-9-4	-40	-70		FRONT	VERT	TOTAL
V	10-8-8	-1147	-1147		FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. G/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

GIRDER TYPE: CP PrimeHip
LEFT SETBACK = 4-0-10
RIGHT SETBACK = 0-0
END SETBACK = 5-10-8
END WALL WIDTH = 2-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDTL LOADS BASED ON 55 % OF GSL

*** NON STANDARD GIRDER ***

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/880 (0.70")
CALCULATED VERT. DEFL.(LL) = L/999 (0.18")
ALLOWABLE DEFL.(TL) = L/880 (0.70")
CALCULATED VERT. DEFL.(TL) = L/891 (0.28")

CSI: TC=0.74/1.00 (C-D:1), BC=0.48/1.00 (I-K:1),
WB=0.84/1.00 (G-I:1), SS=0.74/1.00 (I-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

DRWG NO. TAM 5099-18
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME 286185	TRUSS NAME T1Z	QUANTITY 1	PLY 1	JOB DESC. 43854 TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID: LIPB7XlyxdQBIYCH4AXRGyehXO-IUZx6EHNLmGfh0NWnQKMHU4mUmbIqLWgJEHEAzsYOY

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION
 (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 818 354 1687 622 2284 1686

 PLATE PLACEMENT TOL. = 0.250 inches

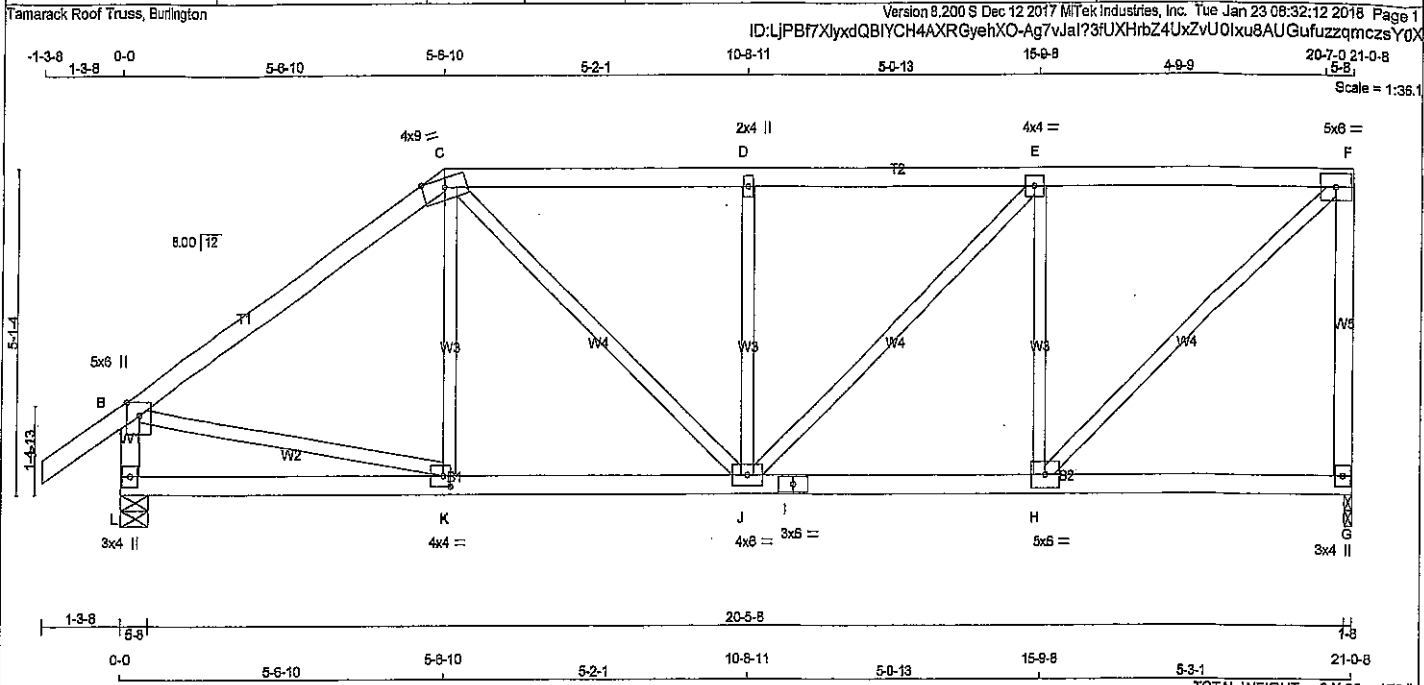
 PLATE ROTATION TOL. = 5.0 Deg.

 JSI GRIP= 0.90 (L) (INPUT = 0.90)
 JSI METAL= 0.87 (J) (INPUT = 1.00)



DWG NO. TAM **S099-18**
 STRUCTURAL
 COMPONENT ONLY

JOB NAME 286185	TRUSS NAME T2	QUANTITY 2	PLY 1	JOB DESC. 43954	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.200 S Dec 12 2017 MTek Industries, Inc. Tue Jan 23 08:32:12 2018 Page 1	ID:LPBF7XlyxdQBIYCH4AXRGyehXO-Ag7vJal73fUXHbZ4UxZvU0Ixu8AUgufuzzqmczsY0X



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF		
C - F	2x4	DRY	No.2	SPF		
G - F	2x4	DRY	No.2	SPF		
L - B	2x4	DRY	No.2	SPF		
L - I	2x4	DRY	No.2	SPF		
I - G	2x4	DRY	No.2	SPF		

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	5.0	6.0	Edge	
C TTWW+m	MT20	4.0	9.0	Edge	
D TMVW+w	MT20	2.0	4.0		
E TMVW-t	MT20	4.0	4.0		
F TMVW-t	MT20	5.0	6.0		
G BMV1+p	MT20	3.0	4.0		
H BMVW-w	MT20	6.0	6.0		
I BS-t	MT20	3.0	6.0		
J BMVW-w	MT20	4.0	6.0		
K BMVW-w	MT20	4.0	4.0	2.00	1.50
L BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT VERT	1181	1181	0	1-8
G L	1297	1297	0	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	970	536 / 0	221 / 0	0 / 0	0 / 0	210 / 0	0 / 0
L	1048	609 / 0	221 / 0	0 / 0	0 / 0	218 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.08 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLP)	MAX. FACTORED CSI (LC)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO	A-B	0 / 32	-84.3	-84.3 0.11 (1)	10.00	K-C	-21 / 202	0.05 (3)
	B-C	-1235 / 0	-84.3	-84.3 0.52 (1)	5.08	B-K	0 / 1045	0.24 (1)
	C-D	-1341 / 0	-84.3	-84.3 0.33 (1)	5.20	H-F	0 / 1394	0.31 (1)
	D-E	-1341 / 0	-84.3	-84.3 0.34 (1)	5.18	C-J	0 / 430	0.10 (1)
	E-F	-1028 / 0	-84.3	-84.3 0.33 (1)	5.74	H-E	-775 / 0	0.30 (1)
	F-G	-1122 / 0	0.0	0.0 0.49 (1)	7.49	J-D	-468 / 0	0.18 (1)
	L-B	-1232 / 0	0.0	0.0 0.13 (1)	7.23	J-E	0 / 432	0.10 (1)
	L-K	0 / 0	-28.0	-28.0 0.21 (3)	10.00			
	K-J	0 / 1026	-28.0	-28.0 0.33 (2)	10.00			
	J-I	0 / 1028	-28.0	-28.0 0.31 (2)	10.00			
	I-H	0 / 1028	-28.0	-28.0 0.31 (2)	10.00			
	H-G	0 / 0	-28.0	-28.0 0.19 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2012, BCBC 2012, ABC 2014
- CSA 068-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.70")
CALCULATED VERT. DEFL. (LL) = L/969 (0.05")
ALLOWABLE DEFL. (TL) = L/360 (0.70")
CALCULATED VERT. DEFL. (TL) = L/999 (0.09")

CSI: TC=0.52/1.00 (B-C:1), BC=0.33/1.00 (J-K:2), WB=0.31/1.00 (F-H:1), SSI=0.21/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1067 822 2264 1858

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.86 (K) (INPUT = 0.90)
JSI METAL= 0.38 (K) (INPUT = 1.00)

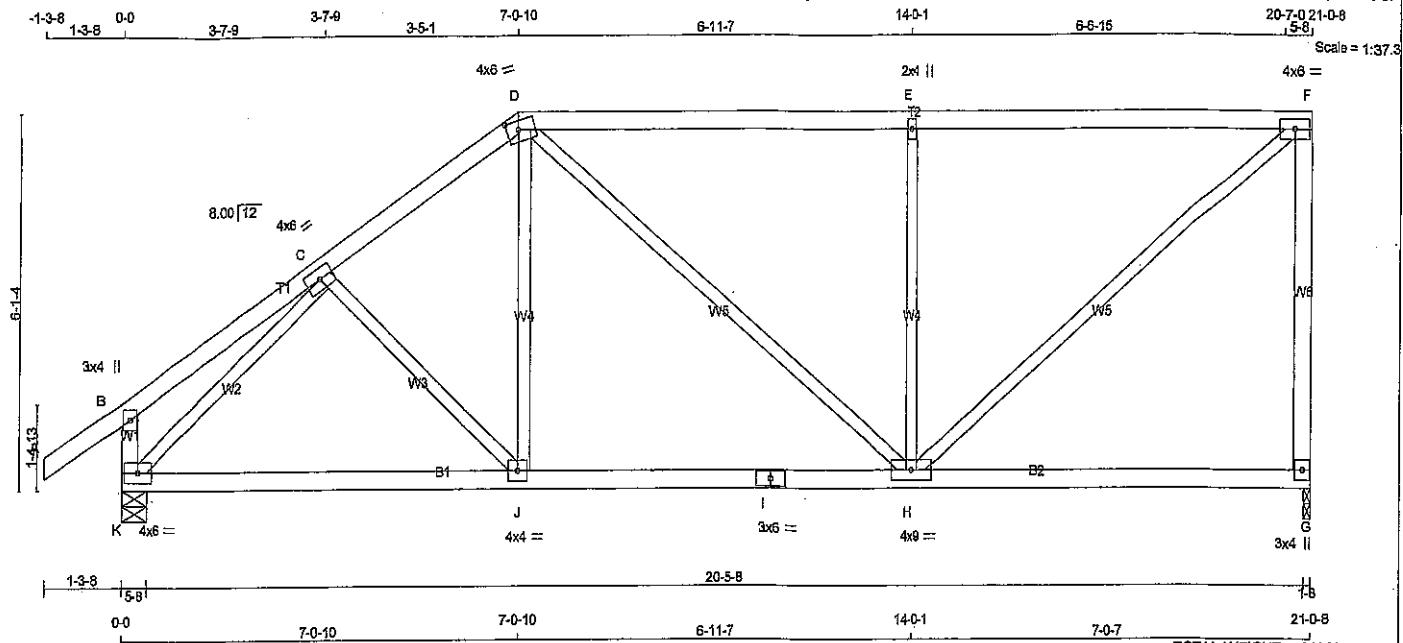


DRWG NO. TAM 5101-12
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
286185	T3	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID: LJPBF7XlyxdQB1YCH4AXRGyehXO-Ag7vJa1?3fUXHrbZ4UxZvU0Eku7RUCKfuzzqmczsyYox



LUMBER					
N. L. G. A. RULES					
CHORDS		SIZE	LUMBER		DESCR
A - D	2x4	DRY	No.2		SPF
D - F	2x4	DRY	No.2		SPF
G - F	2x4	DRY	No.2		SPF
K - B	2x4	DRY	No.2		SPF
K - I	2x4	DRY	No.2		SPF
I - G	2x4	DRY	No.2		SPF

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0			
C	TMVW-t	MT20	4.0	6.0			
D	TTWV-m	MT20	4.0	6.0	1.75	2.50	
E	TMVW-t	MT20	2.0	4.0			
F	TMVW-t	MT20	4.0	6.0			
G	BMV1+p	MT20	3.0	4.0			
H	BMVWV-t	MT20	4.0	9.0			
I	BS-t	MT20	3.0	6.0			
J	BMVW-t	MT20	4.0	4.0			
K	BMVW1-t	MT20	4.0	6.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	DOWN	GROSS REACTION	DOWN	BRG	IN-SX	BRG	IN-SX
G	1181	0	1181	0	0	1-8	1-8		
K	1297	0	1297	0	0	5-8	5-8		

UNFACTORED REACTIONS							
	1ST LCASE	MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	970	538/0	221/0	0/0	0/0	210/0	0/0
K	1048	609/0	221/0	0/0	0/0	219/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.74 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO	FR-TO
A-B	0 / 32	-84.3 -84.3 0.11 (1)	10.00 C-J -47 / 83
B-C	0 / 18	-84.3 -84.3 0.15 (1)	10.00 J-D 0 / 336
C-D	-1192 / 0	-84.3 -84.3 0.18 (1)	5.65 K-C -1409 / 0
D-E	-1059 / 0	-84.3 -84.3 0.78 (1)	4.74 H-F 0 / 1374
E-F	-1059 / 0	-84.3 -84.3 0.78 (1)	4.74 D-H 0 / 103
G-F	-1100 / 0	0.0 0.0 0.79 (1)	7.54 H-E -731 / 0
K-B	-232 / 0	0.0 0.0 0.02 (1)	7.81
K-J	0 / 1010	-28.0 -28.0 0.44 (2)	10.00
J-I	0 / 980	-28.0 -28.0 0.44 (2)	10.00
I-H	0 / 980	-28.0 -28.0 0.44 (2)	10.00
H-G	0 / 0	-28.0 -28.0 0.31 (3)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.70")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.11")
ALLOWABLE DEFL. (TL) = $L/360$ (0.70")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.18")

CSI: TC=0.79/1.00 (F-G:1), BC=0.44/1.00 (H-J:2), WB=0.54/1.00 (C-K:1), SS=0.29/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

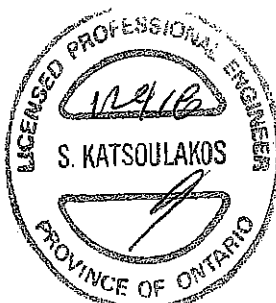
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (F) (INPUT = 0.80)
JSI METAL= 0.34 (C) (INPUT = 1.00)



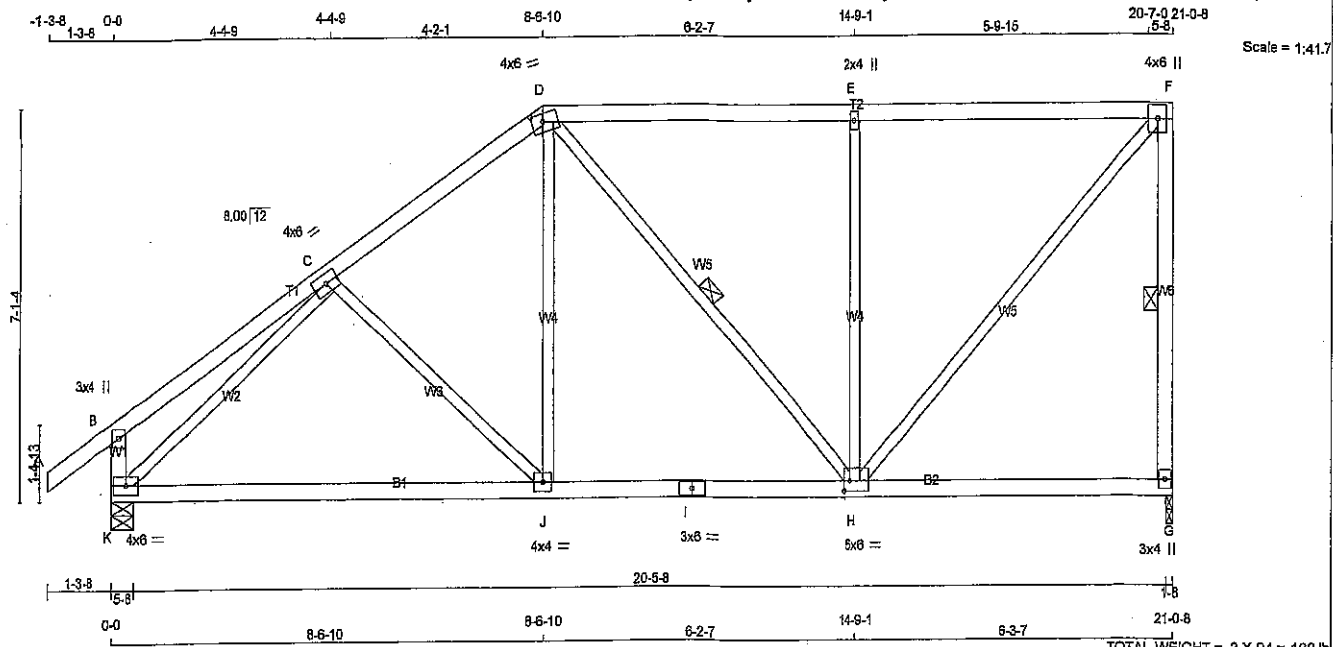
DRWG NO. TAM 5102-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
286185	T4	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID: LjPB7XlyxdQB1YCH4AXRGyehXO-Ag7vJal73fUXHrbZ4UxZvU0Huu5iU8afuzzqmczsyY0X



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
K - B	2x4	DRY	No.2	SPF	
K - I	2x4	DRY	No.2	SPF	
I - G	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW-m	MT20	4.0	6.0	1.75	2.50
E	TMWW-w	MT20	2.0	4.0		
F	TMWW+p	MT20	4.0	6.0		
G	BMV/t+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	6.0	2.25	1.50
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	UPLIFT	IN-SX
G	1181	0	0	1-8
K	1287	0	0	5-8

UNFACTORED REACTIONS

JT	1ST LOOSE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	970	538 / 0	221 / 0	0 / 0	0 / 0	210 / 0	0 / 0
K	1048	809 / 0	221 / 0	0 / 0	0 / 0	219 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.67 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS (I.C)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (I.C)	
FR-TO		FROM TO	LENGTH FR-TO	FR-TO			
A-B	0 / 32	-84.3	-84.3 0.11 (1)	10.00	C-J	-160 / 63	0.10 (1)
B-C	0 / 23	-84.3	-84.3 0.24 (1)	10.00	J-D	0 / 428	0.10 (2)
C-D	-1112 / 0	-84.3	-84.3 0.27 (1)	5.67	K-C	-1392 / 0	0.76 (1)
D-E	-834 / 0	-84.3	-84.3 0.59 (1)	5.68	H-F	0 / 1231	0.28 (1)
E-F	-935 / 0	-84.3	-84.3 0.59 (1)	5.68	D-H	-137 / 0	0.08 (2)
G-F	-1105 / 0	0.0	0.0 0.25 (1)	6.02	H-E	-852 / 0	0.57 (1)
K-B	-254 / 0	0.0	0.0 0.03 (1)	7.81			
K-J	0 / 1025	-28.0	-28.0 0.55 (2)	10.00			
J-I	0 / 910	-28.0	-28.0 0.55 (2)	10.00			
I-H	0 / 910	-28.0	-28.0 0.55 (2)	10.00			
H-G	0 / 0	-28.0	-28.0 0.24 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	48.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.22")
ALLOWABLE DEFL.(TL) = L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = 1/678 (0.37")

CSI: TC=0.59/1.00 (E-F:1), BC=0.55/1.00 (H-J:2), WB=0.78/1.00 (C-K:1), SS=0.25/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MT20	618	354	1667 622 2284 1656

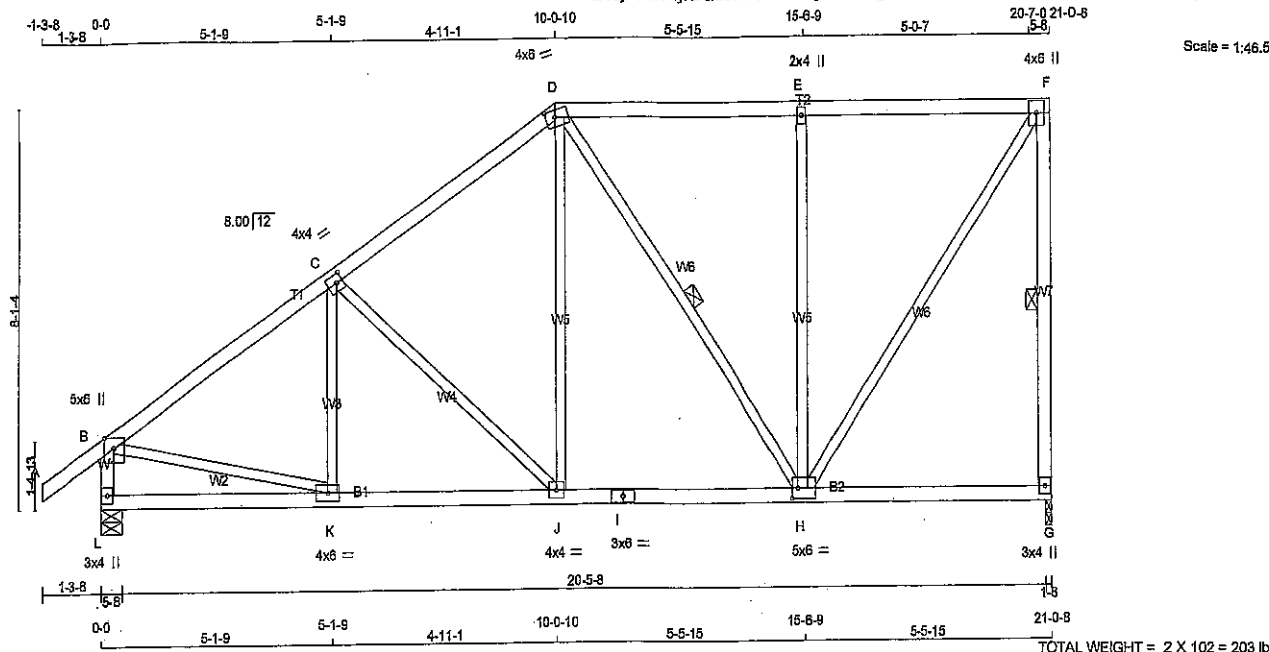
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.67 (H) (INPUT = 0.90)
JSI METAL= 0.33 (C) (INPUT = 1.00)



DRWG NO. TAM 5/03 -18
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 2 X 102 = 203 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
G - F	2x4	DRY	No.2
L - B	2x4	DRY	No.2
L - I	2x4	DRY	No.2
I - G	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMVW+p	MT20	5.0	6.0 Edge
C	TMWW-t	MT20	4.0	4.0 2.00 1.50
D	TTWW-m	MT20	4.0	6.0 1.75 2.50
E	TMWW-w	MT20	2.0	4.0
F	TMVW+p	MT20	4.0	6.0
G	BMV1+p	MT20	3.0	4.0
H	BMWWW-t	MT20	5.0	6.0 2.50 1.50
I	BS-t	MT20	3.0	6.0
J	BMWW-t	MT20	4.0	4.0
K	BMWW-t	MT20	4.0	6.0
L	BMV1+p	MT20	3.0	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
JT	VERT	HORZ	MAXIMUM FACTORED GROSS REACTION
G	1181	0	1181 0 0
L	1297	0	1297 0 0

UNFACTORED REACTIONS						
JT	1ST CASE	MAX/MIN	COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE
G	970	538 / 0	221 / 0	0 / 0	0 / 0	210 / 0
L	1048	609 / 0	221 / 0	0 / 0	0 / 0	219 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.19 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)
FR-TO		FROM	TO
A-B	0 / 32	-84.3	-84.3 0.11 (1)
B-C	-1281 / 0	-84.3	-84.3 0.41 (1)
C-D	-988 / 0	-84.3	-84.3 0.39 (1)
D-E	-666 / 0	-84.3	-84.3 0.44 (1)
E-F	-666 / 0	-84.3	-84.3 0.44 (1)
G-F	-1118 / 0	0.0	0.0 0.33 (1)
L-B	-1297 / 0	0.0	0.0 0.13 (1)
L-K	0 / 0	-28.0	-28.0 0.17 (3)
K-J	0 / 1089	-28.0	-28.0 0.30 (2)
J-I	0 / 803	-28.0	-28.0 0.30 (2)
I-H	0 / 803	-28.0	-28.0 0.30 (2)
H-G	0 / 0	-28.0	-28.0 0.21 (3)

WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC2)
FR-TO		FROM	TO
K-C	-72 / 167	0.04 (3)	
C-J	-391 / 0	0.33 (1)	
J-D	0 / 418	0.09 (2)	
D-H	-238 / 0	0.15 (1)	
H-E	-573 / 0	0.73 (1)	
H-F	0 / 1153	0.26 (1)	
B-K	0 / 1113	0.25 (1)	

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA D88-08
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL)= L/999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL)= L/999 (0.07")

CSI: TC=0.44/1.00 (E-F:1), BC=0.30/1.00 (H-J:2), WB=0.73/1.00 (E-H:1), SSI=0.23/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10
COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 822 2284 1655

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.27 (K) (INPUT = 1.00)



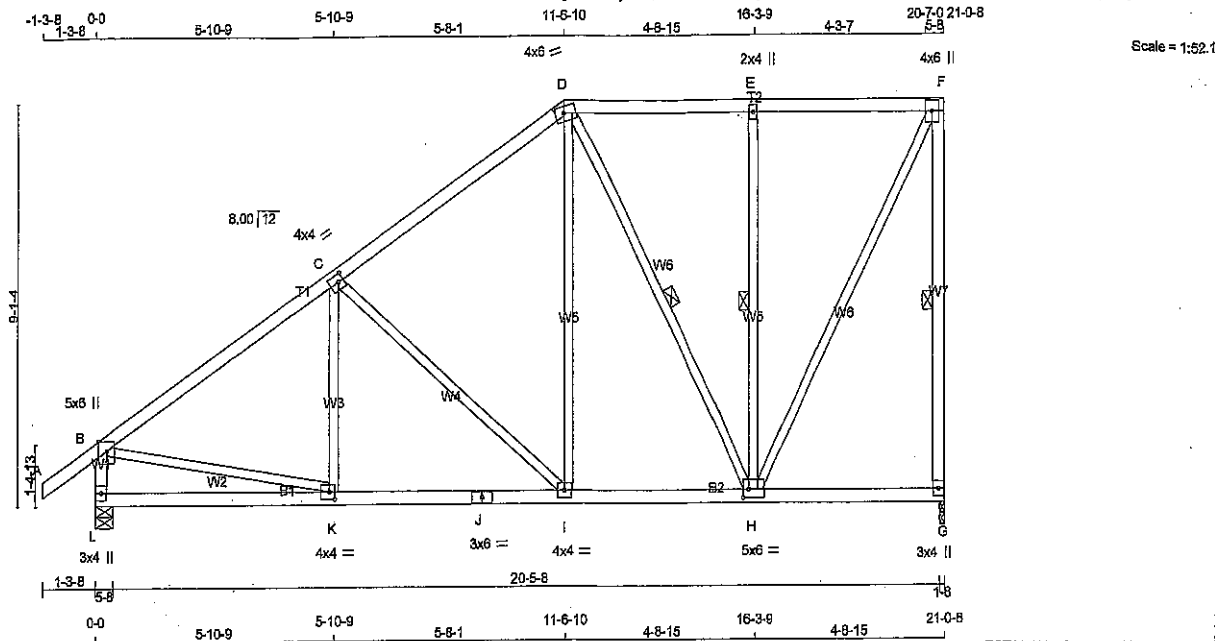
DWG NO. TAW 5104-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43854	DRWG NO.
286185	T6	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 107 = 215 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
G - B	2x4	DRY	No.2	SPF	
L - B	2x4	DRY	No.2	SPF	
L - J	2x4	DRY	No.2	SPF	
J - G	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMVW-l	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	4.0	6.0	1.75	2.50
E	TMVW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMVW-w	MT20	5.0	8.0	2.25	1.50
I	BMVW-l	MT20	4.0	4.0		
J	BS-l	MT20	3.0	8.0		
K	BMVW-l	MT20	4.0	4.0	2.00	1.50
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
G	1181	0	1181	0
L	1297	0	1297	0

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
G	970	538 / 0	221 / 0	0 / 0	0 / 0	210 / 0	0 / 0
L	1048	609 / 0	221 / 0	0 / 0	0 / 0	219 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.24 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED			
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX. (LC)	MAX. UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 32	-84.3	-84.3 0.11 (1)	10.00	K-C	-14 / 231	0.05 (3)
B-C	-1275 / 0	-84.3	-84.3 0.40 (1)	5.24	C-I	-512 / 0	0.80 (1)
C-D	-876 / 0	-84.3	-84.3 0.37 (1)	6.05	I-D	0 / 475	0.11 (2)
D-E	-527 / 0	-84.3	-84.3 0.24 (1)	6.25	D-H	-379 / 0	0.27 (1)
E-F	-527 / 0	-84.3	-84.3 0.24 (1)	6.25	H-E	-493 / 0	0.27 (1)
G-F	-1128 / 0	0.0	0.0 0.43 (1)	5.98	H-F	0 / 1105	0.25 (1)
L-B	-1230 / 0	0.0	0.0 0.13 (1)	7.23	B-K	0 / 1105	0.25 (1)
L-K	0 / 0	-28.0	-28.0 0.24 (3)	10.00			
K-J	0 / 1087	-28.0	-28.0 0.37 (2)	10.00			
J-I	0 / 1087	-28.0	-28.0 0.37 (2)	10.00			
I-H	0 / 708	-28.0	-28.0 0.23 (2)	10.00			
H-G	0 / 0	-28.0	-28.0 0.15 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 3.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 48.1	PSF

SPACING = 240 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.70")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.05")
ALLOWABLE DEFL.(TL) = $L/360$ (0.70")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.09")

CSI: TC=0.43/1.00 (F-G:1), BC=0.37/1.00 (I-K:2), WB=0.60/1.00 (C-I:1), SSI=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

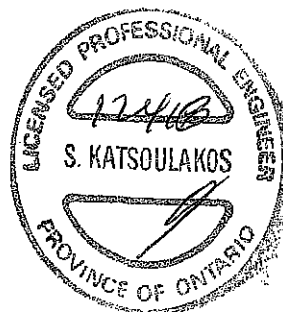
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	518	354	1687
	522	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (K) (INPUT = 0.90)
JSI METAL= 0.41 (K) (INPUT = 1.00)

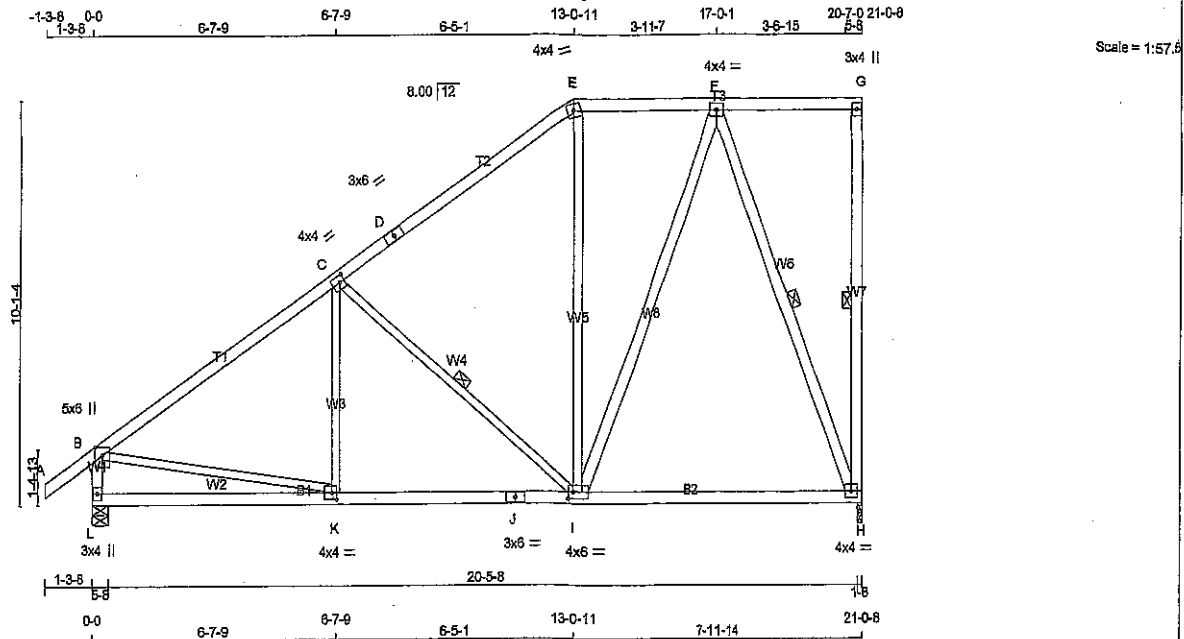


DRWG NO. TAM 5/05 -8
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
286185	T7	4	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID: LJPB7XlyxdQBIYCH4AXRGyehXO-eshHXWjeqzOu?AleCSoRIZThISgDdNp7dJOJ2zsY0VW



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
H - G	2x4	DRY	No.2	SPF	
L - B	2x4	DRY	No.2	SPF	
L - J	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	

ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
I - F	2x4	DRY	No.2	SPF	
F - H	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	8.0	Edge		
C	TMWW-I	MT20	4.0	4.0	2.00	1.50	
D	TS-I	MT20	3.0	6.0			
E	TTW-m	MT20	4.0	4.0			
F	TMWW-I	MT20	4.0	4.0			
G	TMV+p	MT20	3.0	4.0			
H	BMVW-I	MT20	4.0	4.0			
I	BMWW-I	MT20	4.0	6.0	2.00	1.50	
J	BS-I	MT20	3.0	6.0			
K	BMWW-I	MT20	4.0	4.0	2.00	1.50	
L	BMV1+p	MT20	3.0	4.0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	SNOW	GROSS REACTION	DOWN	BRG	IN-SX	BRG	IN-SX
H	1181	0	1181	0	0	1-8	1-8		
L	1297	0	1297	0	0	5-8	5-8		

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN. COMPONENT REACTIONS		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL		
H	970	538 / 0	221 / 0	0 / 0	0 / 0	210 / 0	0 / 0		
L	1048	609 / 0	221 / 0	0 / 0	0 / 0	219 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.07 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-H, C-I, F-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 32	-84.3 -84.3	0.11 (1)	10.00	K-C	0 / 250	0.06 (3)
B-C	-1253 / 0	-84.3 -84.3	0.52 (1)	5.07	C-I	-908 / 0	0.30 (1)
C-D	-770 / 0	-84.3 -84.3	0.48 (1)	6.14	I-E	0 / 187	0.04 (3)
D-E	-770 / 0	-84.3 -84.3	0.48 (1)	6.14	I-F	0 / 586	0.09 (1)
E-F	-908 / 0	-84.3 -84.3	0.18 (1)	6.25	F-H	-1040 / 0	0.62 (1)
F-G	0 / 0	-84.3 -84.3	0.22 (1)	10.00	B-K	0 / 1067	0.24 (1)
H-G	-128 / 0	0.0 0.0	0.06 (1)	6.25			
L-B	-1219 / 0	0.0 0.0	0.13 (1)	7.26			
L-K	0 / 0	-28.0 -28.0	0.26 (3)	10.00			
K-J	0 / 1073	-28.0 -28.0	0.51 (2)	10.00			
J-I	0 / 1073	-28.0 -28.0	0.51 (2)	10.00			
I-H	0 / 395	-28.0 -28.0	0.43 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/999 (0.15")
ALLOWABLE DEFL.(TL) = L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/999 (0.25")

CSI: TC=0.52/1.00 (B-C:1), BC=0.51/1.00 (I-K:2), WB=0.62/1.00 (F-H:1), SS=0.22/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

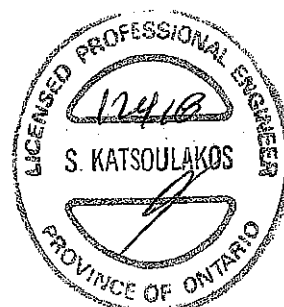
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1657 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

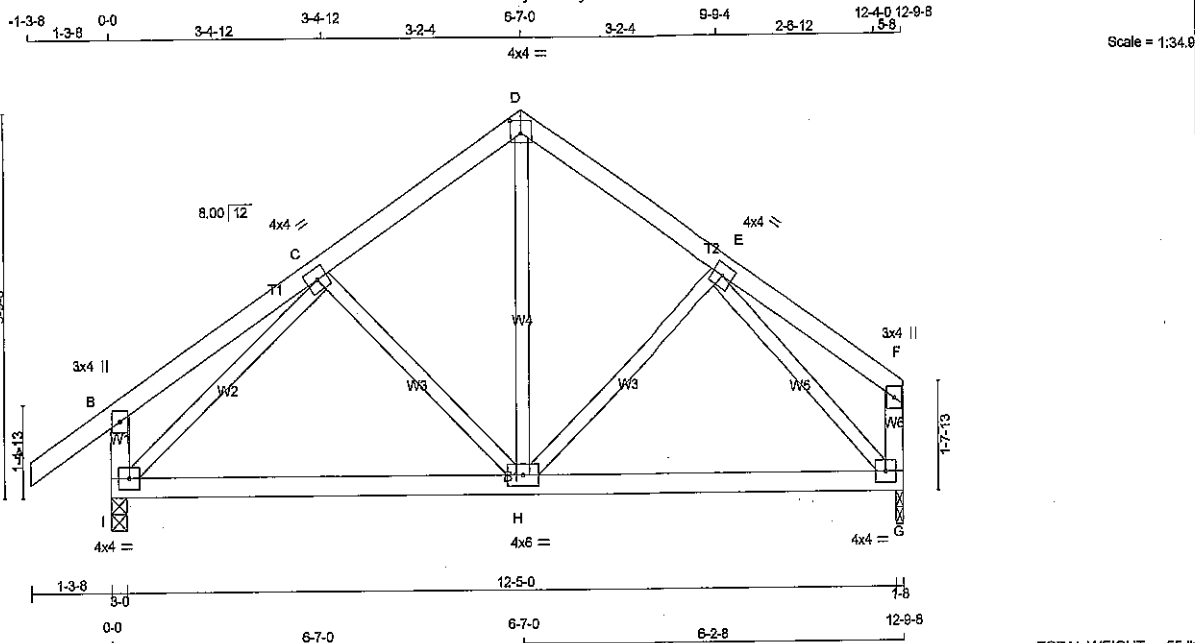
JSI GRIP= 0.86 (K) (INPUT = 0.90)
JSI METAL= 0.40 (K) (INPUT = 1.00)



DWG NO. TAM5006-18
STRUCTURAL
COMPONENT ONLY

Tamarack Roof Truss, Burlington

ID: LJPBf7XlyxdQB1YCH4AXRGyehXO-KqXF0IFBYd4uj6?Eshwz3maDsFQewbC2VIQxXzsYNq



TOTAL WEIGHT = 55 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
I	834	0	834	0	3-0	3-0
G	718	0	718	0	1-8	1-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	668	397 / 0	134 / 0	0 / 0	0 / 0	138 / 0	0 / 0
G	589	327 / 0	134 / 0	0 / 0	0 / 0	128 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)		
FR-TO		FROM	TO		FR-TO				
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	C-H	-137 / 40	0.05 (1)	
B-C	0 / 17	-84.3	-84.3	0.14 (1)	10.00	H-D	0 / 380	0.09 (2)	
C-D	-551 / 0	-84.3	-84.3	0.11 (1)	6.25	H-E	-77 / 61	0.03 (1)	
D-E	-549 / 0	-84.3	-84.3	0.10 (1)	6.25	I-C	-784 / 0	0.26 (1)	
E-F	0 / 17	-84.3	-84.3	0.13 (1)	10.00	E-G	-751 / 0	0.23 (1)	
I-B	-223 / 0	0.0	0.0	0.02 (1)	7.81				
G-F	-91 / 0	0.0	0.0	0.01 (1)	7.81				
I-H	0 / 541	-28.0	-28.0	0.39 (2)	10.00				
H-G	0 / 499	-28.0	-28.0	0.39 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.43")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.06")
ALLOWABLE DEFL.(TL) = $L/360$ (0.43")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.10")

CSI: TC=0.14/1.00 (B-C:1), BC=0.39/1.00 (H-I:2), WB=0.26/1.00 (C-I:1), SSI=0.16/1.00 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 L.S BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2264 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.70 (E) (INPUT = 0.90)
JSI METAL = 0.27 (C) (INPUT = 1.00)



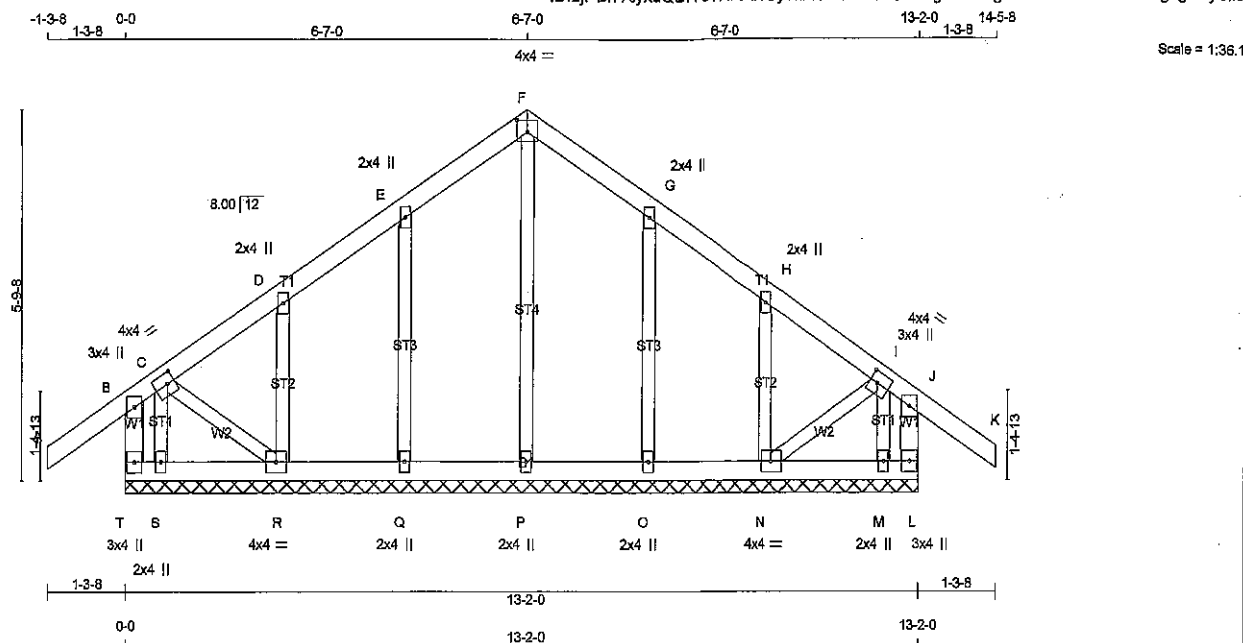
DWG NO. TAM5067-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43854	DRWG NO.
286139	G8	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID: LJPB7XlyxdQB1YCH4AXRGyehXO-0NAAPQsvoXgH2Cz6gU?9x1eBw132n?Timgxg2Vyeif3



Scale = 1:36.1

TOTAL WEIGHT = 2 X 59 = 119 lb

LUMBER	CHORDS	SIZE	LUMBER	DESCR
N. L. G. A. RULES				
T - B	2x4	DRY	No.2	SPF
A - F	2x4	DRY	No.2	SPF
F - K	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

GABLE STUDS SPACED AT 2'-0" OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TMW+w	MT20	2.0	4.0		
F	TTW+p	MT20	4.0	4.0	2.25	2.00
I	TMWW-t	MT20	4.0	4.0	2.00	1.50
J	TMV+p	MT20	3.0	4.0		
L	BMV1+p	MT20	3.0	4.0		
M, O, P, Q, S						
N	BMW1+w	MT20	2.0	4.0		
R	BMWW1-t	MT20	4.0	4.0		
N	BMWW1-t	MT20	4.0	4.0		
T	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	MAX. UNBRACED LENGTH	FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	FR-TO
T-B	-249 / 0	0.0	0.0	0.03 (1)	7.81	P-F	-115 / 0	0.08 (1)	
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	Q-E	-191 / 0	0.06 (1)	
B-C	-61 / 0	-84.3	-84.3	0.11 (1)	6.25	R-D	-184 / 0	0.03 (1)	
C-D	-31 / 0	-84.3	-84.3	0.04 (1)	6.25	S-C	-30 / 0	0.00 (1)	
D-E	-26 / 0	-84.3	-84.3	0.05 (1)	6.25	O-G	-191 / 0	0.08 (1)	
E-F	-33 / 0	-84.3	-84.3	0.05 (1)	6.25	N-H	-184 / 0	0.03 (1)	
F-G	-33 / 0	-84.3	-84.3	0.05 (1)	6.25	M-I	-30 / 0	0.00 (1)	
G-H	-26 / 0	-84.3	-84.3	0.05 (1)	6.25	C-R	0 / 35	0.01 (1)	
H-I	-31 / 0	-84.3	-84.3	0.04 (1)	6.25	N-I	0 / 35	0.01 (1)	
I-J	-61 / 0	-84.3	-84.3	0.11 (1)	6.25				
J-K	0 / 32	-84.3	-84.3	0.11 (1)	10.00				
L-J	-249 / 0	0.0	0.0	0.03 (1)	7.81				
T-S	0 / 0	-28.0	-28.0	0.02 (2)	10.00				
S-R	0 / 0	-28.0	-28.0	0.02 (2)	10.00				
R-Q	0 / 24	-28.0	-28.0	0.03 (2)	10.00				
Q-P	0 / 20	-28.0	-28.0	0.02 (2)	10.00				
P-O	0 / 20	-28.0	-28.0	0.02 (2)	10.00				
O-N	0 / 24	-28.0	-28.0	0.03 (2)	10.00				
N-M	0 / 0	-28.0	-28.0	0.02 (2)	10.00				
M-L	0 / 0	-28.0	-28.0	0.02 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD	=	46.1	PSF	

SPACING = 24.0 IN. G/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11 (A-B:1), BC=0.03 (N-O:2), WB=0.06 (F-P:1), SSI=0.09 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

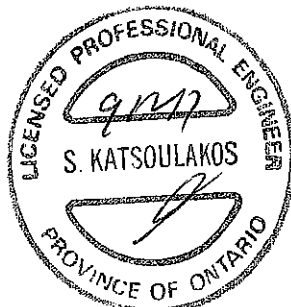
PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX MIN
MT20	618	354	1667 822 2284 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (E) (INPUT = 0.90)

JSI METAL= 0.05 (E) (INPUT = 1.00)



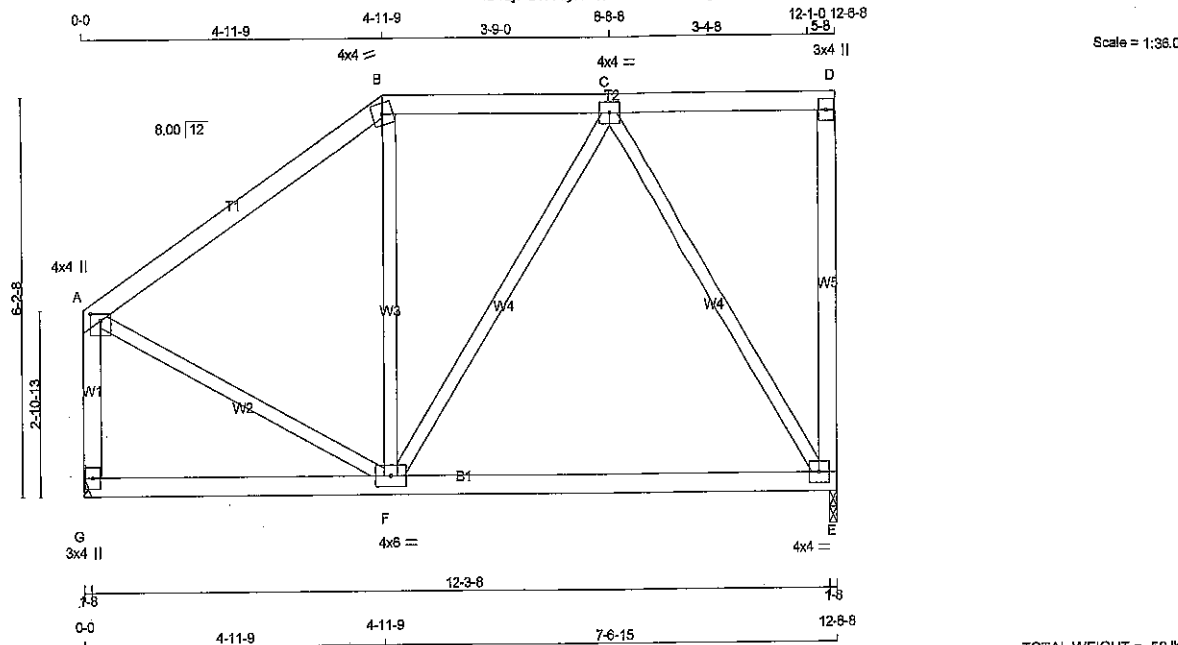
DWG NO. TAM 46122-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43554	DRWG NO.
286140	T9A	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID: LJPBf7XlyxdQB1YCH4AXRGyehXO-KqXFOIFBYd4uj8?Eshwz3mXgsFQesBC2VIQxXzsYNq



TOTAL WEIGHT = 59 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	No.2
A - B	2x4	DRY	No.2
B - D	2x4	DRY	No.2
E - D	2x4	DRY	No.2
G - A	2x4	DRY	No.2
G - E	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00
B	TTW-m	MT20	4.0	4.0		
C	TMVW-i	MT20	4.0	4.0		
D	TMV+p	MT20	3.0	4.0		
E	BMVW1-i	MT20	4.0	4.0		
F	BMVW1-i	MT20	4.0	4.0		
G	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	UP
E	704	0	704	0
G	704	0	704	0

UNFACTORED REACTIONS

JT	1ST CASE	MAX	MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE	PERM. LIVE
E	578	321 / 0	132 / 0	0 / 0
G	578	321 / 0	132 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	WEBS
	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO	FROM TO	LENGTH FR-TO
A-B	-463 / 0	-84.3 -84.3 0.37 (1)
B-C	-383 / 0	-84.3 -84.3 0.20 (1)
C-D	0 / 0	-84.3 -84.3 0.20 (1)
E-D	-123 / 0	0.0 0.0 0.09 (1)
G-A	-662 / 0	0.0 0.0 0.10 (1)
G-F	0 / 0	-28.0 -28.0 0.36 (3)
F-E	0 / 321	-28.0 -28.0 0.39 (2)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD	=	46.1	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/380 (0.42")
CALCULATED VERT. DEFL.(LL)= L/969 (0.11")
ALLOWABLE DEFL.(TL)= L/380 (0.42")
CALCULATED VERT. DEFL.(TL)= L/803 (0.19")

CSI: TC=0.37/1.00 (A-B:1), BC=0.39/1.00 (E-F:2),
WB=0.54/1.00 (C-E:1), SS=0.17/1.00 (E-F:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	618	354	1667

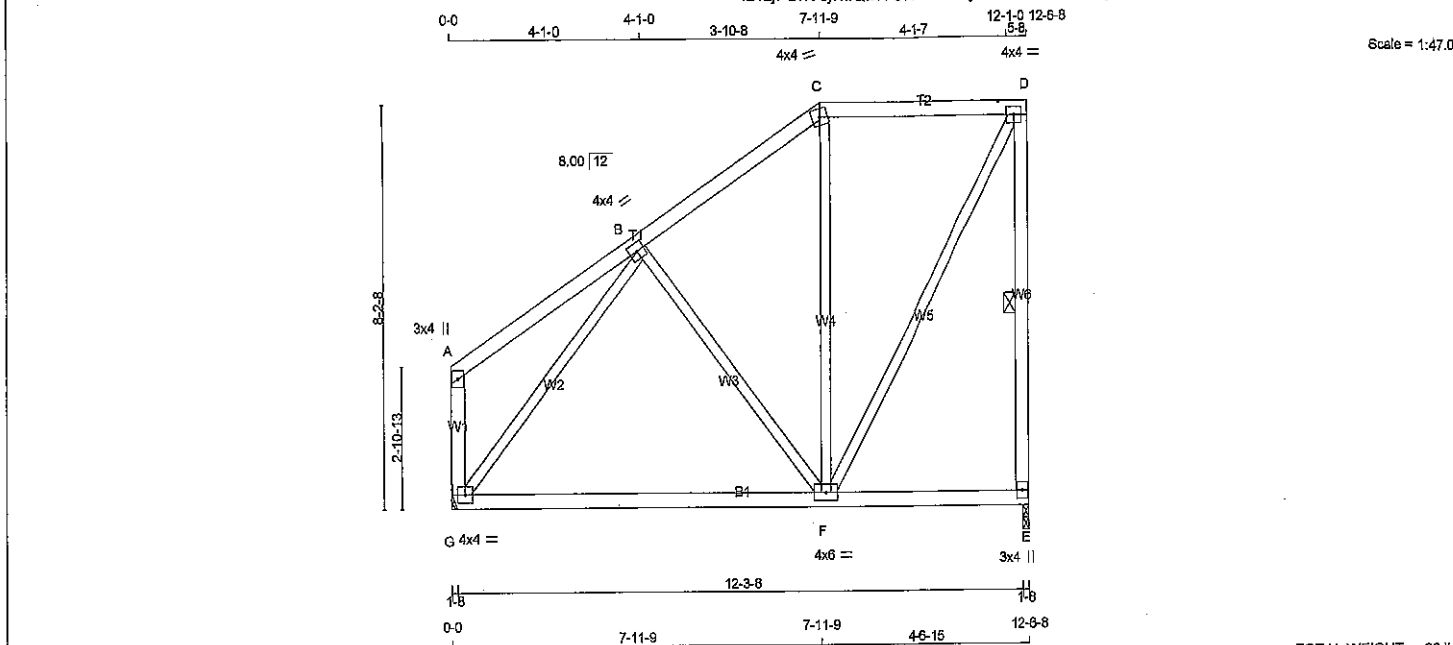
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.56 (F) (INPUT = 0.90)
JSI METAL= 0.14 (C) (INPUT = 1.00)



DWG NO. TAM 506B-13
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 66 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - A	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table ts in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	4.0		
C	TTW-m	MT20	4.0	4.0		
D	TMVW-t	MT20	4.0	4.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMVWW-t	MT20	4.0	6.0		
G	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
E	704	0	704	0	1-8	1-8
G	704	0	704	0	HANGER BY OTHERS	MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS				
		SNOW	LIVE	PERM. LIVE	WIND	DEAD
E	578	321 / 0	132 / 0	0 / 0	0 / 0	125 / 0
G	576	321 / 0	132 / 0	0 / 0	0 / 0	125 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-E.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED				MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
		VERT. LOAD (PLF)	LC1 (LC)	MAX. UNBRACED LENGTH					
FR-TO		FROM	TO			FR-TO			
A-B	0/23	-84.3	-84.3	0.22 (1)	10.00	B-F	-148/44	0.12 (1)	
B-C	-355/0	-84.3	-84.3	0.17 (1)	6.25	F-C	-137/81	0.18 (1)	
C-D	-275/0	-84.3	-84.3	0.23 (1)	6.25	F-D	0/558	0.13 (1)	
E-D	-678/0	0.0	0.0	0.21 (1)	6.25	G-B	-809/0	0.46 (1)	
G-A	-126/0	0.0	0.0	0.02 (1)	7.81				
G-F	0/368	-28.0	-28.0	0.44 (2)	10.00				
F-E	0/0	-28.0	-28.0	0.40 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 3.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.42")
CALCULATED VERT. DEFL.(LL) = L/958 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.42")
CALCULATED VERT. DEFL.(TL) = L/574 (0.26")

CSI: TC=0.23/1.00 (C-D:1), BC=0.44/1.00 (F-G:2), WB=0.46/1.00 (B-G:1), SSI=0.18/1.00 (F-G:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP(DRY)		SHEAR		SECTION
	(PSI)		(PLI)		(PLI)
	MAX	MIN	MAX	MIN	MAX MIN
MT20	618	354	1667	822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

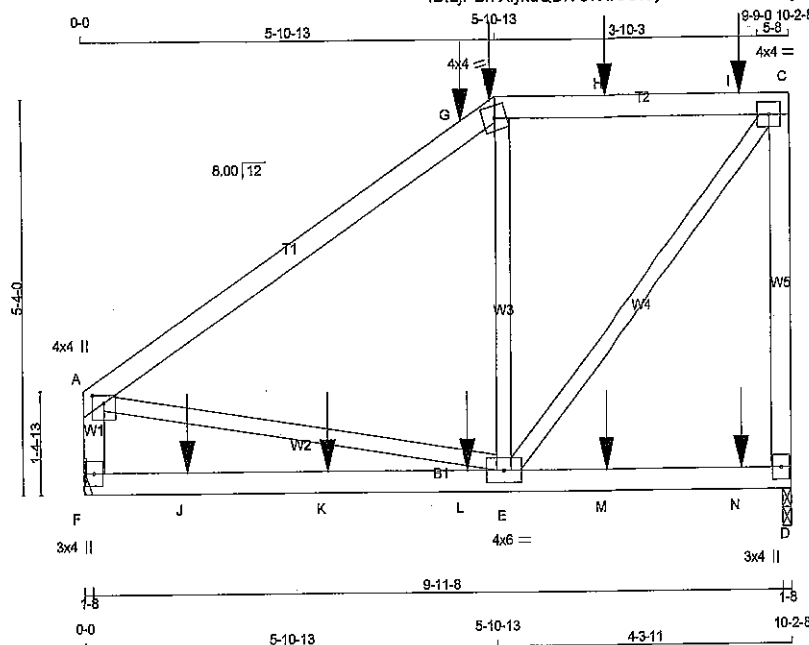
JSI GRIP= 0.72 (B) (INPUT = 0.90)
JSI METAL= 0.21 (B) (INPUT = 1.00)



Tamarack Roof Truss, Burlington

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ID: LJPB7XlyxdQBIYCH4AXRGyehXO-o05db5GOyrtxWshCoZC9WHJemGcaNPGLG81_TzssYNp



LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCOR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x6	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
A	TMVW+p	MT20	4.0	4.0 1.25 2.00
B	TTW-m	MT20	4.0	4.0
C	TMVW-t	MT20	4.0	4.0
D	BMV1+p	MT20	3.0	4.0
E	BMVWW-t	MT20	4.0	8.0
F	BMV1+p	MT20	3.0	4.0

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 179.8 lbs FACTORED DOWN AT: 5-10-12, 49.7 lbs FACTORED DOWN AT 5-5-12, AND 25.2 lbs FACTORED DOWN AT 7-5-12, AND 36.6 lbs FACTORED DOWN AT 9-5-12 ON TOP CHORD, AND 26.6 lbs FACTORED DOWN AT 1-5-12, 26.6 lbs FACTORED DOWN AT 3-5-12, 26.6 lbs FACTORED DOWN AT 5-5-12, AND 26.6 lbs FACTORED DOWN AT 7-5-12, AND 33.2 lbs FACTORED DOWN AT 9-5-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	HORZ	GROSS REACTION	DOWN	HORZ	BRG	IN-SX	BRG	IN-SX
D	800	0	800	0	0	1-8	1-8		
F	717	0	717	0	0	HANGER BY OTHERS			
						MIN. SEAT SIZE: 1-6			

UNFACTORED REACTIONS		1ST LCASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL		
D	850	372 / 0	140 / 0	0 / 0	0 / 0	137 / 0	0 / 0		
F	555	327 / 0	134 / 0	0 / 0	0 / 0	128 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX CSI (LC)	MAX. MEMB. UNBRAC LENGTH FR-TO	MAX. FORCE (LBS)
FR-TO		FROM TO			
A-G	-528 / 0	-84.3 -84.3	0.80 (1)	6.25	E-B -360 / 90
G-B	-528 / 0	-84.3 -84.3	0.60 (1)	6.25	E-C 0 / 687
B-H	-436 / 0	-84.3 -84.3	0.35 (1)	6.25	A-E 0 / 456
H-I	-436 / 0	-84.3 -84.3	0.35 (1)	6.25	
I-C	-436 / 0	-84.3 -84.3	0.35 (1)	6.25	
D-C	-751 / 0	0.0 0.0	0.38 (1)	7.81	
F-A	-632 / 0	0.0 0.0	0.07 (1)	7.81	
F-J	0 / 0	-28.0 -28.0	0.33 (3)	10.00	
J-K	0 / 0	-28.0 -28.0	0.33 (3)	10.00	
K-L	0 / 0	-28.0 -28.0	0.33 (3)	10.00	
L-E	0 / 0	-28.0 -28.0	0.33 (3)	10.00	
E-M	0 / 0	-28.0 -28.0	0.33 (3)	10.00	
M-N	0 / 0	-28.0 -28.0	0.33 (3)	10.00	
N-D	0 / 0	-28.0 -28.0	0.33 (3)	10.00	

FACTORED CONCENTRATED LOADS (LBS)		FACE		DIR		TYPE	
JT	LOC.	LC1	MAX-	MAX+			
B	5-10-12	-15	-18		FRONT	VERT	DEAD
B	5-10-12	-164	-164		FRONT	VERT	SNOW
G	5-5-12	-50	-50		FRONT	VERT	TOTAL
H	5-5-12	-25	-25		FRONT	VERT	TOTAL
I	9-5-12	-37	-37		FRONT	VERT	TOTAL
J	1-5-12	-15	-27		FRONT	VERT	TOTAL
K	3-5-12	-15	-27		FRONT	VERT	TOTAL
L	5-5-12	-15	-27		FRONT	VERT	TOTAL
M	7-5-12	-15	-27		FRONT	VERT	TOTAL
N	9-5-12	-19	-33		FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDTL. USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CSI: TC=0.60/1.00 (A-B:1), BC=0.33/1.00 (E-F:3),
WB=0.17/1.00 (C-E:1), SS=0.21/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

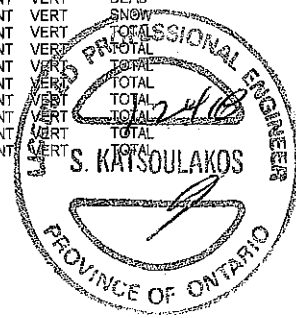
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (E) (INPUT = 0.90)
JSI METAL= 0.18 (E) (INPUT = 1.00)

DRWG NO. TAM 5070-18
STRUCTURAL
COMPONENT ONLY

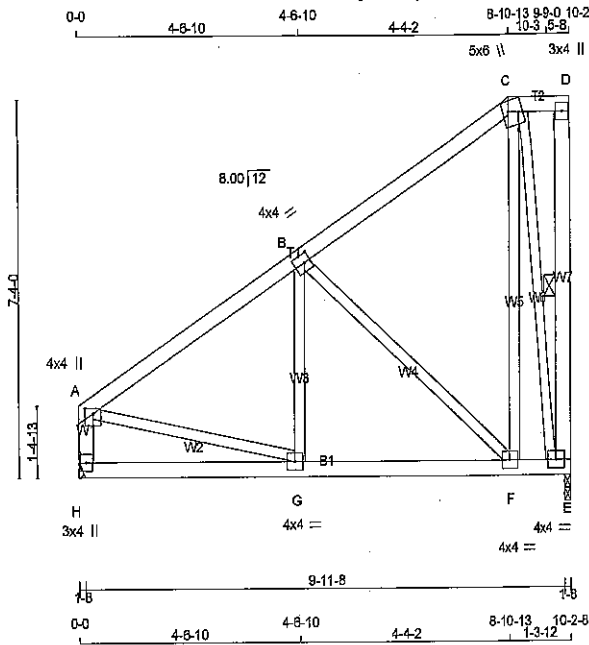


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
286140	T12	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID: LJPB7XlyxdQBIYCH4AXRGyehXO-o05db5GOyrbxWshCoZC9WHJkoGehNK7LG81_TzssYNp



Scale = 1:44.0

TOTAL WEIGHT = 57 lb

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW+p	MT20	4.0	4.0	1.25 2.00
B	TMWW-t	MT20	4.0	4.0	2.00 1.50
C	TTWW+m	MT20	5.0	6.0	2.50 1.50
D	TMV+p	MT20	3.0	4.0	
E	BMVW1-t	MT20	4.0	4.0	
F	BMWW-t	MT20	4.0	4.0	
G	BMWW-t	MT20	4.0	4.0	
H	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
E	573	0	573	0	0	1-8	1-8		
H	573	0	573	0	0				

UNFACTORED REACTIONS							
1ST LCASE	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	470	261 / 0	107 / 0	0 / 0	0 / 0	102 / 0	0 / 0
E	470	261 / 0	107 / 0	0 / 0	0 / 0	102 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-E.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX	MEMB.	FORCE (LBS)	MAX. UNBRAC LENGTH	CS1 (LC)
FR-TO		FROM TO	CS1 (LC)	FR-TO			
A-B	-474 / 0	-84.3 -84.3	0.21 (1)	G-B	0 / 212	6.25	0.05 (3)
B-C	-132 / 0	-84.3 -84.3	0.21 (1)	B-F	-445 / 0	6.25	0.28 (1)
C-D	0 / 0	-84.3 -84.3	0.02 (1)	F-C	0 / 372	10.00	0.08 (1)
E-D	-55 / 0	0.0 0.0	0.01 (1)	C-E	-514 / 0	8.25	0.50 (1)
H-A	-523 / 0	0.0 0.0	0.05 (1)	A-G	0 / 427	7.81	0.10 (1)
H-G	0 / 0	-28.0 -28.0	0.15 (3)			10.00	
G-F	0 / 415	-28.0 -28.0	0.20 (2)			10.00	
F-E	0 / 94	-28.0 -28.0	0.06 (3)			10.00	

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.21/1.00 (A-B:1), BC=0.20/1.00 (F-G:2), WB=0.50/1.00 (C-E:1), SS=0.15/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

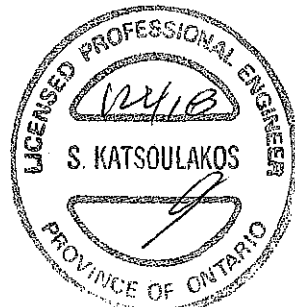
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.49 (G) (INPUT = 0.90)
JSI METAL= 0.16 (G) (INPUT = 1.00)

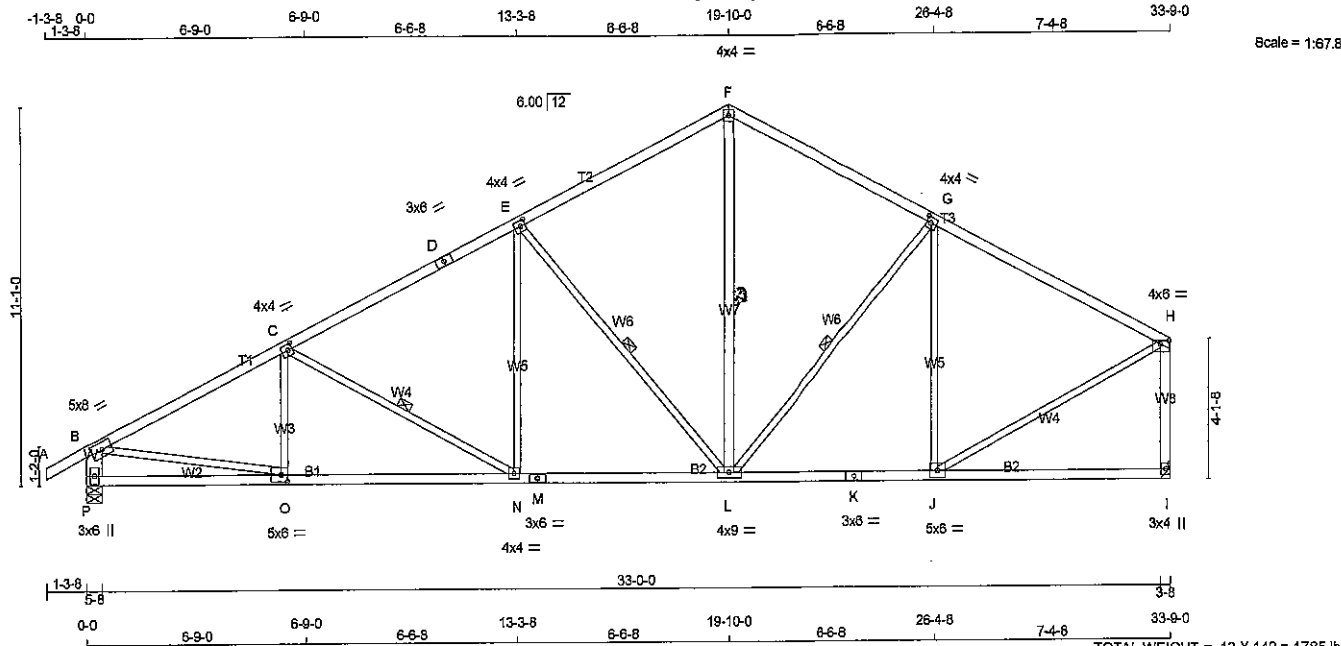


DRWG NO. TAM 5071-08
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43984	DRWG NO.
286139	T13A	12	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID: LJPB7XlyxdQBHYCH4AXRGyehXO-NKz3S8w2cTJa9zr3S1bKe4Lv52edS5XRvyeRjijyefl



TOTAL WEIGHT = 12 X 149 = 1785 lb

LUMBER	SIZE	DRY	LUMBER	DESCR.
N. L. G. A. RULES				
CHORDS				
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
P - B	2x6	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
L - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE				
B TMVW-t	MT20	5.0	8.0	
C, E, G				
C TMVW-t	MT20	4.0	4.0	2.00 1.75
D TS-t	MT20	3.0	6.0	
F TTW-p	MT20	4.0	4.0	
H TMVW-p	MT20	4.0	6.0	1.25 3.00
I BMV1+p	MT20	3.0	4.0	
J BMVW-t	MT20	5.0	6.0	
K BS-t	MT20	3.0	6.0	
L BMVW-t	MT20	4.0	9.0	
M BS-t	MT20	3.0	6.0	
N BMVW-t	MT20	4.0	4.0	
O BMVW-t	MT20	5.0	6.0	2.50 2.25
P BMV1+p	MT20	3.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
P	2009	0	2009	0
I	1894	0	1894	0

UNFACTORED REACTIONS

	1ST LCASE	MAX/MIN. COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
P	1633	933 / 0	354 / 0	0 / 0	0 / 0	346 / 0
I	1555	863 / 0	354 / 0	0 / 0	0 / 0	338 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.68 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-N, E-L, G-L. *F-L*

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	UNBRAC LENGTH FR-TO	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
A-B	0 / 26	-84.3	-84.3	0.11 (1)	10.00	O-C	-155 / 182	0.04 (1)	
B-C	-2810 / 0	-84.3	-84.3	0.56 (1)	3.68	C-N	-494 / 0	0.23 (1)	
C-D	-2346 / 0	-84.3	-84.3	0.60 (1)	3.95	N-E	0 / 517	0.12 (2)	
D-E	-2346 / 0	-84.3	-84.3	0.60 (1)	3.95	E-L	-998 / 0	0.71 (1)	
E-F	-1644 / 0	-84.3	-84.3	0.51 (1)	4.50	L-F	0 / 1033	0.17 (1)	
F-G	-1651 / 0	-84.3	-84.3	0.50 (1)	4.43	L-G	-145 / 0	0.10 (1)	
G-H	-1692 / 0	-84.3	-84.3	0.66 (1)	4.30	J-G	-575 / 0	0.66 (1)	
P-B	-1940 / 0	0.0	0.0	0.13 (1)	7.22	B-O	0 / 2560	0.58 (1)	
I-H	-1810 / 0	0.0	0.0	0.51 (1)	6.22	J-H	0 / 1735	0.39 (1)	
P-O	0 / 0	-28.0	-28.0	0.28 (3)	10.00				
O-N	0 / 2535	-28.0	-28.0	0.57 (2)	10.00				
N-M	0 / 2102	-28.0	-28.0	0.51 (2)	10.00				
M-L	0 / 2102	-28.0	-28.0	0.51 (2)	10.00				
L-K	0 / 1541	-28.0	-28.0	0.53 (2)	10.00				
K-J	0 / 1541	-28.0	-28.0	0.53 (2)	10.00				
J-I	0 / 0	-28.0	-28.0	0.35 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 3.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.12")
CALCULATED VERT. DEFL.(LL) = L/999 (0.16")
ALLOWABLE DEFL.(TL) = L/360 (1.12")
CALCULATED VERT. DEFL.(TL) = L/999 (0.27")

CSI: TC=0.66 (G-H:1), BC=0.57 (N-O:2),
WB=0.71 (E-L:1), SSI=0.26 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

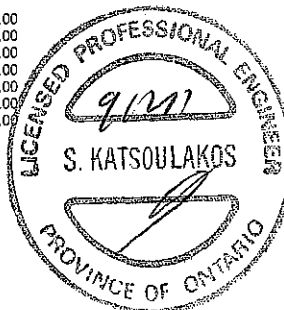
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (O) (INPUT = 0.90)
JSI METAL= 0.64 (M) (INPUT = 1.00)

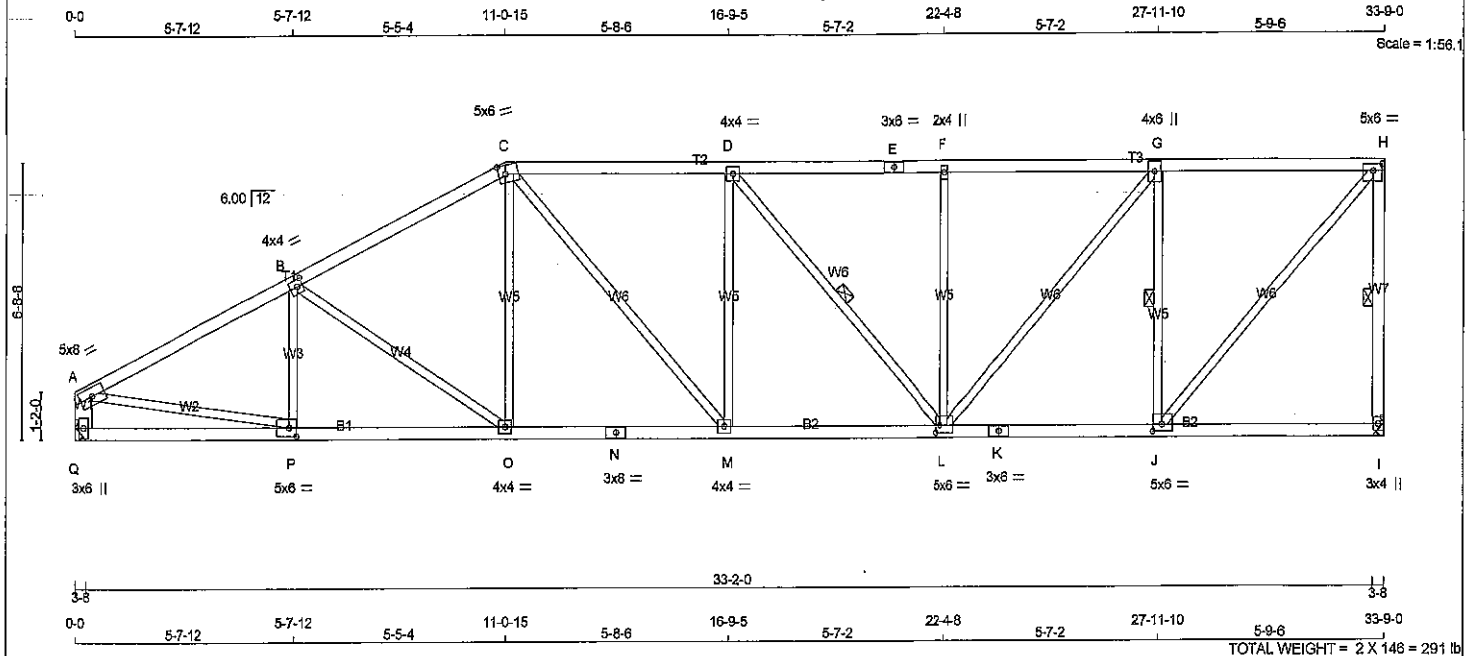


DWG NO. TAM 46159-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
286139	T14	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID: LJPBf7XlyxdQB1YCH4AXRGyehXO-NKz3S8w2ctJa9zr351bKe4LvE2fnS8gRvyeRjJyef



TOTAL WEIGHT = 2 X 146 = 291 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - H	2x4	DRY	No.2
I - H	2x4	DRY	No.2
Q - A	2x6	DRY	No.2
Q - N	2x4	DRY	No.2
N - K	2x4	DRY	No.2
K - I	2x4	DRY	No.2

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	8.0		
B	TMVW-t	MT20	4.0	4.0	2.00	1.75
C	TTWW-m	MT20	5.0	6.0	2.50	2.00
D	TMVW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMVW-w	MT20	2.0	4.0		
G	TMVW-t	MT20	4.0	6.0		
H	TMVW-t	MT20	5.0	6.0	2.00	2.75
I	BMV1+p	MT20	3.0	4.0		
J	BMVW-t	MT20	5.0	6.0	2.00	2.75
K	BS-t	MT20	3.0	6.0		
L	BMVW-w	MT20	5.0	6.0	2.25	1.50
M	BMVW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	6.0		
O	BMVW-t	MT20	4.0	4.0		
P	BMVW-t	MT20	5.0	6.0	2.50	2.25
Q	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	HORZ	UPLIFT	IN-SX
I	1894	0	1894	0	HANGER BY OTHERS
Q	1894	0	1894	0	HANGER BY OTHERS

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
I	1555	863 / 0	354 / 0	0 / 0	0 / 0	337 / 0	0 / 0
Q	1555	863 / 0	354 / 0	0 / 0	0 / 0	337 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.57 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF H-I, G-J, D-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX. FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX. CS1 (LC)	UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX. CS1 (LC)	UNBRAC LENGTH
FR-TO		FROM	TO			FR-TO			
A-B	-2790 / 0	-84.3	-84.3	0.65 (1)	3.57	P-B	-218 / 114	0.05 (1)	
B-C	-2493 / 0	-84.3	-84.3	0.80 (1)	3.78	B-O	-372 / 0	0.30 (1)	
C-D	-2545 / 0	-84.3	-84.3	0.57 (1)	3.78	O-C	0 / 409	0.09 (2)	
D-E	-2262 / 0	-84.3	-84.3	0.53 (1)	4.00	A-P	0 / 2548	0.57 (1)	
E-F	-2262 / 0	-84.3	-84.3	0.53 (1)	4.00	J-H	0 / 2201	0.50 (1)	
F-G	-2262 / 0	-84.3	-84.3	0.54 (1)	3.99	C-M	0 / 501	0.11 (1)	
G-H	-1473 / 0	-84.3	-84.3	0.47 (1)	4.83	J-G	-1453 / 0	0.42 (1)	
H-I	-1829 / 0	0.0	0.0	0.38 (1)	4.95	M-D	-215 / 112	0.18 (1)	
Q-A	-1826 / 0	0.0	0.0	0.12 (1)	7.38	L-G	0 / 1200	0.27 (1)	
						D-L	-431 / 0	0.21 (1)	
						L-F	-433 / 0	0.32 (1)	
Q-P	0 / 0	-28.0	-28.0	0.20 (3)	10.00				
P-O	0 / 2517	-28.0	-28.0	0.50 (1)	10.00				
O-N	0 / 2213	-28.0	-28.0	0.45 (1)	10.00				
N-M	0 / 2213	-28.0	-28.0	0.45 (1)	10.00				
M-L	0 / 2546	-28.0	-28.0	0.50 (1)	10.00				
L-K	0 / 1473	-28.0	-28.0	0.40 (2)	10.00				
K-J	0 / 1473	-28.0	-28.0	0.40 (2)	10.00				
J-I	0 / 0	-28.0	-28.0	0.23 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.8	PSF
	DL = 3.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.12")
CALCULATED VERT. DEFL.(LL) = L/999 (0.17")
ALLOWABLE DEFL.(TL) = L/360 (1.12")
CALCULATED VERT. DEFL.(TL) = L/999 (0.28")

CSI: TC=0.65 (A-B:1), BC=0.50 (L-M:1), WB=0.57 (A-P:1), SS=0.23 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1667 622 2284 1658

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.89 (P) (INPUT = 0.90)
JSI METAL= 0.64 (N) (INPUT = 1.00)

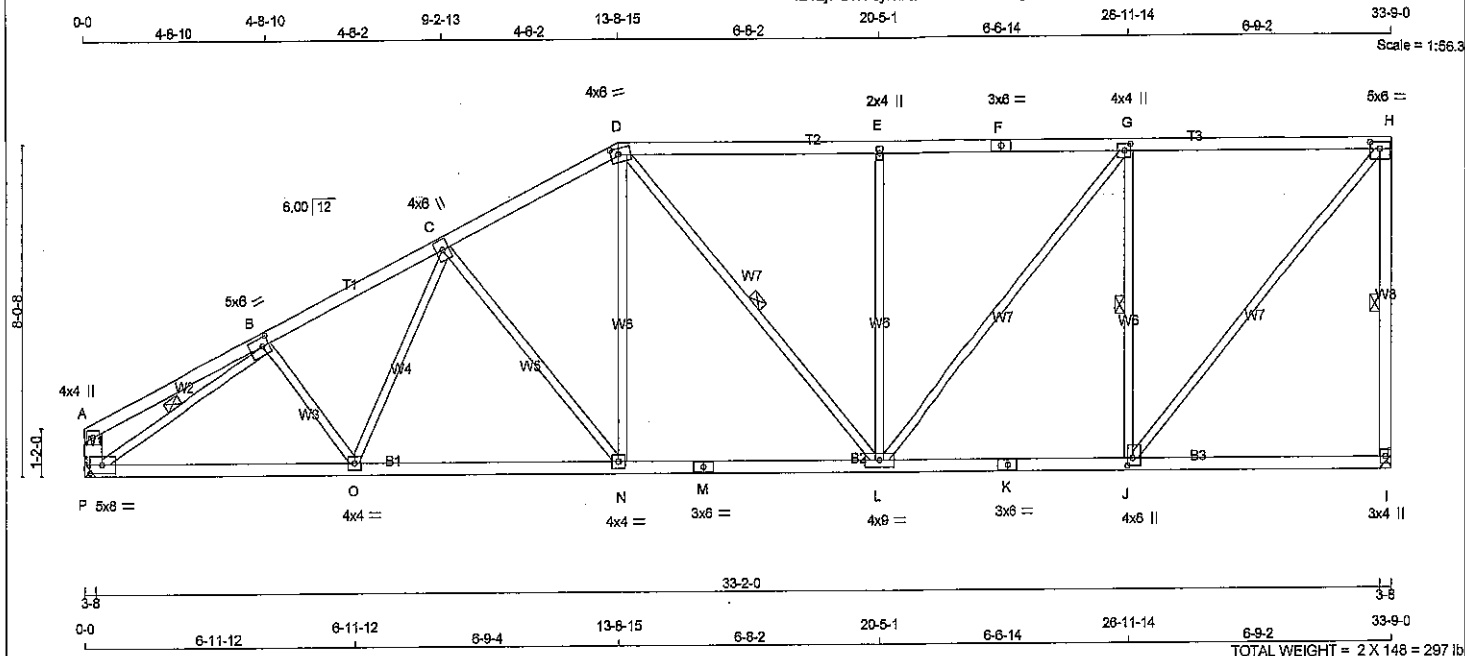


DRWG NO. TAM 46162-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
286139	T15	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID: LJPB7XlyxdQBIYCH4AXRGyehXO-NKz3S6w2cTJa9zr3S1bKe4Lvg2dLS5(RvyeRj)jyef



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
I - H	2x4	DRY	No.2
P - A	2x6	DRY	No.2
P - M	2x4	DRY	No.2
M - K	2x4	DRY	No.2
K - I	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
SPF			
DRY, SEASONED LUMBER.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER							
BEARINGS							
	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG			
JT	VERT	HORZ	DOWN	UP	IN-SX	IN-SX	
I	1894	0	1894	0	0		HANGER BY OTHERS
P	1894	0	1894	0	0		HANGER BY OTHERS
							MIN. SEAT SIZE: 3-8

UNFACTORED REACTIONS							
1ST LCASE	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	1555	863 / 0	354 / 0	0 / 0	0 / 0	337 / 0	0 / 0
P	1555	863 / 0	354 / 0	0 / 0	0 / 0	337 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.90 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF H-I, B-P, D-L, G-J.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING			
TOTAL LOAD CASES: (4)			
CHORDS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)
FR-TO		FROM TO	
A-B	0 / 114	-84.3 -84.3	0.22 (1)
B-C	-2708 / 0	-84.3 -84.3	0.37 (1)
C-D	-2255 / 0	-84.3 -84.3	0.33 (1)
D-E	-2028 / 0	-84.3 -84.3	0.87 (1)
E-F	-2028 / 0	-84.3 -84.3	0.69 (1)
F-G	-2028 / 0	-84.3 -84.3	0.69 (1)
G-H	-1385 / 0	-84.3 -84.3	0.61 (1)
H-I	-1818 / 0	0.0 0.0	0.53 (1)
P-A	-183 / 0	0.0 0.0	0.01 (1)
F-O	0 / 2407	-28.0 -28.0	0.59 (2)
O-N	0 / 2346	-28.0 -28.0	0.59 (2)
N-M	0 / 2008	-28.0 -28.0	0.47 (2)
M-L	0 / 2008	-28.0 -28.0	0.47 (2)
L-K	0 / 1385	-28.0 -28.0	0.47 (2)
K-J	0 / 1385	-28.0 -28.0	0.47 (2)
J-I	0 / 0	-28.0 -28.0	0.31 (3)

WEBS
MEMB. MAX. FACTORED FORCE (LBS) MAX. FACTORED CSI (LC)
FR-TO
B-O 0 / 158 0.04 (3)
O-C 0 / 285 0.05 (2)
C-N -535 / 0 0.53 (1)
N-D 0 / 597 0.13 (1)
P-B -2819 / 0 0.65 (1)
J-H 0 / 2108 0.47 (1)
D-L -18 / 30 0.01 (3)
J-G -1376 / 0 0.58 (1)
L-E -807 / 0 0.75 (1)
L-G 0 / 994 0.22 (1)

DESIGN CRITERIA	
SPECIFIED LOADS:	
TOP CH.	LL = 25.6 PSF
	DL = 3.0 PSF
BOT CH.	LL = 10.5 PSF
	DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF	
SPACING = 24.0 IN. C/C	

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.12")
CALCULATED VERT. DEFL.(LL)= L/ 989 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (1.12")
CALCULATED VERT. DEFL.(TL)= L/ 989 (0.27")

CSI: TC=0.69 (E-G:1), BC=0.59 (O-P:2), WB=0.75 (E-L:1), SI=0.26 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

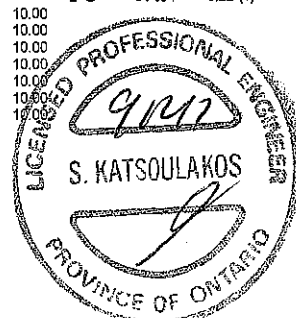
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1687 822 2284 1856

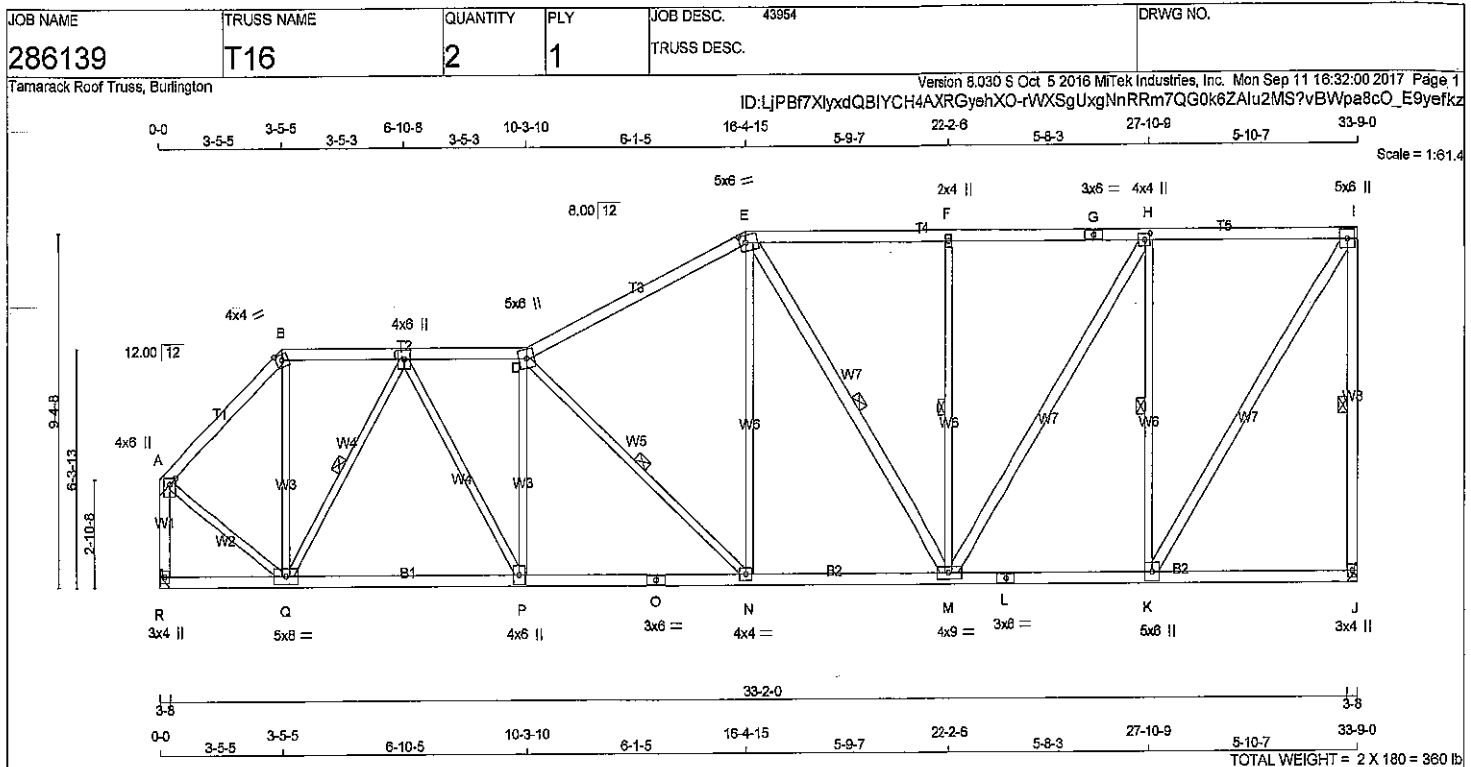
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.72 (B) (INPUT = 1.00)



DRWG NO. TAM 46161-17
STRUCTURAL
COMPONENT ONLY



LUMBER										DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER										DESIGN CRITERIA									
N. L. G. A. RULES																				SPECIFIED LOADS:									
CHORDS SIZE LUMBER DESCR.										BEARINGS										TOP CH. LL = 25.6 PSF									
A - B 2x4 DRY No.2 SPF										FACTORED GROSS REACTION										DL = 3.0 PSF									
B - D 2x4 DRY No.2 SPF										GROSS REACTION										BOT CH. LL = 10.5 PSF									
D - E 2x4 DRY No.2 SPF										DOWN HORZ UPLIFT										DL = 7.0 PSF									
E - G 2x4 DRY No.2 SPF										1694 0 0										TOTAL LOAD = 46.1 PSF									
G - I 2x4 DRY No.2 SPF										R 1694 0 1694 0 0																			
J - I 2x4 DRY No.2 SPF																													
R - A 2x4 DRY No.2 SPF																													
R - O 2x4 DRY No.2 SPF																													
O - L 2x4 DRY No.2 SPF																													
L - J 2x4 DRY No.2 SPF																													
ALL WEBS 2x3 DRY No.2 SPF										UNFACTORED REACTIONS										SPACING = 24.0 IN. C/C									
EXCEPT										1ST LCASE MAX/MIN COMPONENT REACTIONS										LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12									
K - I 2x4 DRY No.2 SPF										JT COMBINED SNOW LIVE PERM. LIVE WIND DEAD SOIL										THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010									
E - M 2x4 DRY No.2 SPF										J 1555 863 / 0 354 / 0 0 / 0 0 / 0 337 / 0 0 / 0										THIS DESIGN COMPLIES WITH:									
M - H 2x4 DRY No.2 SPF										R 1555 863 / 0 354 / 0 0 / 0 0 / 0 337 / 0 0 / 0										- PART 9 OF OBC 2012, BCBC 2012, ABC 2014									
DRY: SEASONED LUMBER.																				- CSA 086-09									
																				- TPIC 2011									
																				(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD									
																				ALLOWABLE DEFL.(LL)= L/360 (1.12")									
																				CALCULATED VERT. DEFL.(LL) = L/999 (0.14")									
																				ALLOWABLE DEFL.(TL)= L/360 (1.12")									
																				CALCULATED VERT. DEFL.(TL) = L/999 (0.23")									
																				CSI: TC=0.76 (I-J:1), BC=0.51 (N-P:2), WB=0.84 (H-K:1), SS=0.23 (H-I:1)									
																				DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10									
																				COMP=1.10 SHEAR=1.10 TENS=1.10									
																				COMPANION LIVE LOAD FACTOR = 0.50									
																				TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.									
																				NAIL VALUES									
																				PLATE GRIP(DRY) SHEAR SECTION									
																				(PSI) (PLI) (PLI)									
																				MAX MIN MAX MIN MAX MIN									
																				MT20 618 354 1667 822 2284 1656									
																				PLATE PLACEMENT TOL. = 0.250 inches									
																				PLATE ROTATION TOL. = 5.0 Deg.									
																				JSI GRIP= 0.90 (H) (INPUT = 0.90)									
																				JSI METAL= 0.68 (O) (INPUT = 1.00)									

PLATES (table is in inches)									
JT	TYPE	PLATES	W	LEN	Y	X			
A	TMVW+p	MT20	4.0	6.0	Edge				
B	TTW-m	MT20	4.0	4.0	Edge				
C	TMWW+t	MT20	4.0	6.0					
D	TTWW+m	MT20	5.0	8.0					
E	TTWW-m	MT20	5.0	6.0	2.25	2.00			
F	TMW+w	MT20	2.0	4.0					
G	TS-t	MT20	3.0	6.0					
H	TMWW+t	MT20	4.0	4.0	2.00	1.75			
I	TMVW+p	MT20	5.0	6.0					
J	BMVt+p	MT20	3.0	4.0					
K	BMWW+t	MT20	5.0	6.0					
L	BS-t	MT20	3.0	6.0					
M	BMWWW-t	MT20	4.0	9.0					
N	BMWW-t	MT20	4.0	4.0					
O	BS-t	MT20	3.0	6.0					
P	BMWW+t	MT20	4.0	6.0					
Q	BMWWW-t	MT20	5.0	6.0					
R	BMVt+p	MT20	3.0	4.0					

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

CHORDS										WEBS									
MAX. FACTORED					FACTORED					MAX. FACTORED					MAX. FACTORED				
MEMB. FORCE (LBS)					VERT. LOAD LC1 MAX (PLF) CSI (LC)					MEMB. FORCE (LBS) CSI (LC)					MEMB. FORCE (LBS) CSI (LC)				
FR-TO										FR-TO									
A-B -1422 / 0										Q-B 0 / 745 0.17 (1)									
B-C -1014 / 0										Q-C -1477 / 0 0.48 (1)									
C-D -2253 / 0										C-P 0 / 1088 0.24 (1)									
D-E -1971 / 0										P-D -767 / 0 0.49 (1)									
E-F -1820 / 0										D-N -702 / 0 0.36 (1)									
F-G -1820 / 0										N-E 0 / 559 0.15 (2)									
G-H -1820 / 0										A-Q 0 / 1229 0.28 (1)									
H-I -1052 / 0										K-I 0 / 1939 0.31 (1)									
J-I -1829 / 0										E-M -278 / 0 0.17 (1)									
R-A -1857 / 0										K-H -1443 / 0 0.84 (1)									
										M-F -525 / 0 0.30 (1)									
										M-H 0 / 1071 0.17 (1)									
R-Q 0 / 0																			
Q-P 0 / 1728																			
P-D 0 / 2267																			
O-N 0 / 2267																			
N-M 0 / 1770																			
M-L 0 / 1052																			
L-K 0 / 1052																			
K-J 0 / 0																			

9/12/17

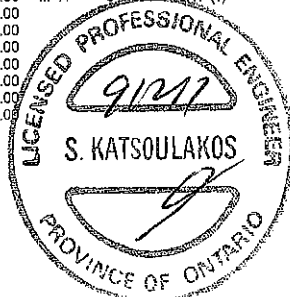
S. KATSOUKAKOS

PROVINCE OF ONTARIO

DWG NO. TAN 46162-17

STRUCTURAL

COMPONENT ONLY



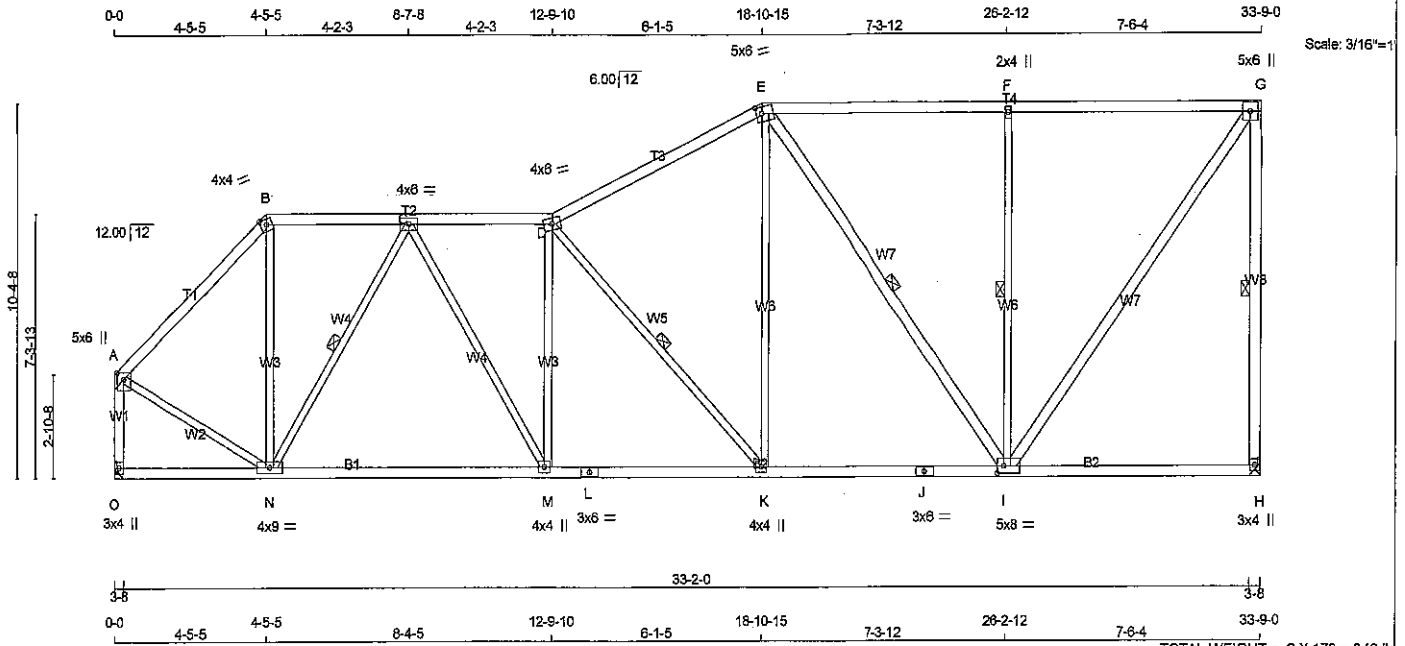
DWG NO. TAN 46162-17
 STRUCTURAL
 COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
286139	T17	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2
B - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
H - G	2x4	DRY	No.2
O - A	2x4	DRY	No.2
O - L	2x4	DRY	No.2
L - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
E - I	2x4	DRY	No.2
I - G	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW+p	MT20	5.0	6.0	2.25 2.50
B	TTW-m	MT20	4.0	4.0	Edge
C	TMWW-t	MT20	4.0	6.0	
D	TTWW-m	MT20	4.0	6.0	
E	TTWW-m	MT20	5.0	8.0	2.25 2.00
F	TMW+w	MT20	2.0	4.0	
G	TMVW+p	MT20	5.0	8.0	
H	BMV1+p	MT20	3.0	4.0	
I	BMWW-t	MT20	5.0	8.0	2.50 2.25
J	BS-t	MT20	3.0	6.0	
K	BMWW-t	MT20	4.0	4.0	
L	BS-t	MT20	3.0	6.0	
M	BMWW-t	MT20	4.0	4.0	
N	BMWW-t	MT20	4.0	9.0	
O	BMV1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT		REQD	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	BRG	BRG
H	1894	0	1894	0	0				
O	1894	0	1894	0	0				

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
H	COMBINED	SNOW	LIVE			
H	1555	863 / 0	354 / 0	0 / 0	0 / 0	337 / 0
O	1555	863 / 0	354 / 0	0 / 0	0 / 0	337 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.17 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-H, C-N, D-K, E-I, F-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO				FR-TO	
A-B	-1519 / 0	-84.3	-84.3 0.38 (1)	A-N	0 / 748
B-C	-1081 / 0	-84.3	-84.3 0.28 (1)	N-B	0 / 748
C-D	-2154 / 0	-84.3	-84.3 0.31 (1)	C-M	0 / 824
D-E	-1749 / 0	-84.3	-84.3 0.70 (1)	M-D	-504 / 0
E-F	-1184 / 0	-84.3	-84.3 0.91 (1)	D-K	-901 / 0
F-G	-1184 / 0	-84.3	-84.3 0.92 (1)	K-E	0 / 847
H-G	-1812 / 0	0.0	0.0 0.67 (1)	E-I	-891 / 0
O-A	-1859 / 0	0.0	0.0 0.29 (1)	I-F	-777 / 0
				I-G	0 / 1958
O-N	0 / 0	-28.0	-28.0 0.31 (3)	A-N	0 / 1224
N-M	0 / 1740	-28.0	-28.0 0.51 (2)		
M-L	0 / 2161	-28.0	-28.0 0.57 (2)		
L-K	0 / 2161	-28.0	-28.0 0.57 (2)		
K-J	0 / 1573	-28.0	-28.0 0.58 (2)		
J-I	0 / 1573	-28.0	-28.0 0.58 (2)		
I-H	0 / 0	-28.0	-28.0 0.40 (3)		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 089-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.12")
CALCULATED VERT. DEFL.(LL) = L/999 (0.16")
ALLOWABLE DEFL.(TL) = L/360 (1.12")
CALCULATED VERT. DEFL.(TL) = L/999 (0.26")

CSI: TC=0.97 (G-H:1), BC=0.58 (I-K:2), WB=0.63 (E-I:1), SSI=0.30 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

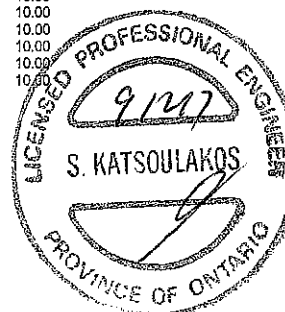
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.60 (L) (INPUT = 1.00)



DRWG NO. TAM 46163-87
STRUCTURAL
COMPONENT ONLY



CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286141	T18	1	1	TRUSS DESC.	

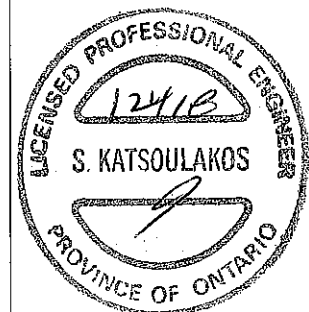
Tamarack Roof Truss, Burlington

Version 8.200 S Dec 12 2017 MitTek Industries, Inc. Tue Jan 23 08:14:56 2018 Page 2

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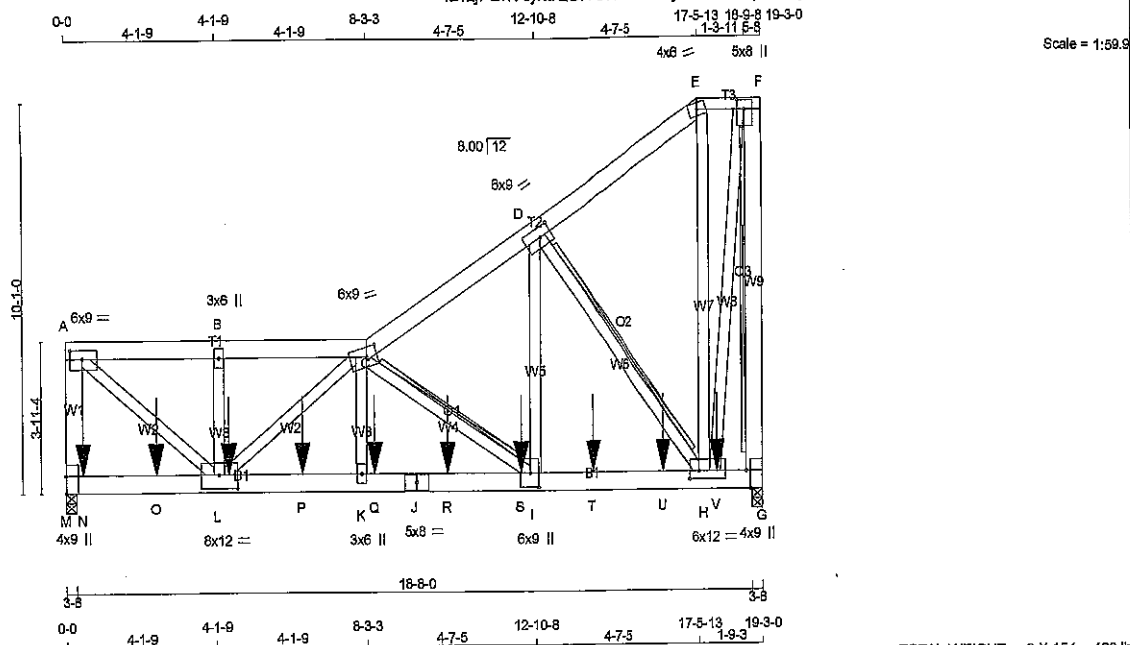
FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
Q	4-6-4	9	1	14	FRONT	VERT	TOTAL
R	6-4-12	9	1	14	FRONT	VERT	TOTAL
S	8-8-4	-105	-105	—	FRONT	VERT	TOTAL
T	10-6-4	-133	-133	—	FRONT	VERT	TOTAL
U	12-6-4	-133	-133	—	FRONT	VERT	TOTAL
V	14-6-4	-133	-133	—	FRONT	VERT	TOTAL
W	16-6-4	-133	-133	—	FRONT	VERT	TOTAL
X	18-6-4	-137	-137	—	FRONT	VERT	TOTAL



DWG NO. TAM 5062 -18
STRUCTURAL
COMPONENT ONLY

DWG NO. TAM 5063-18
STRUCTURAL
COMPONENT ONLY



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
M - A	2x6	DRY No.2	SPF
A - C	2x6	DRY No.2	SPF
C - E	2x6	DRY No.2	SPF
E - F	2x4	DRY No.2	SPF
G - F	2x6	DRY No.2	SPF
M - J	2x6	DRY 2100F 1.8E	SPF
J - G	2x6	DRY 2100F 1.8E	SPF
ALL WEBS	2x4	DRY No.2	SPF
DRY: SEASONED LUMBER.			

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
M-A 2	8	SIDE (258.1)
A-C 2	12	TOP
C-E 2	12	TOP
E-F 2	12	TOP
F-G 1	12	TOP
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
M-J 2	9	SIDE (683.7)
J-G 2	8	SIDE (555.0)
WEBS: (0.122"x3") SPIRAL NAILS		
2x4 1	8	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

<u>PLATES (table is in inches)</u>						
JT	TYPE	PLATES	W	LEN	Y	X
A	TMWV-t	MT20	6.0	9.0	2.75	4.00
B	TMWV-w	MT20	3.0	6.0		
C	TTWWV-m	MT20	6.0	9.0	3.00	3.50
D	TMWV-t	MT20	6.0	9.0	3.00	3.75
E	TTW-m	MT20	4.0	6.0		
F	TMWV+p	MT20	5.0	8.0	2.75	2.25
G	BMV1+p	MT20	4.0	9.0	Edge	1.50
H	BMWVW-t	MT20	8.0	12.0	2.50	3.25
I	BMWVW-t	MT20	6.0	9.0	4.50	2.75
J	BS-t	MT20	5.0	8.0		
K	BMWV-w	MT20	3.0	6.0		
L	BMWVW-t	MT20	8.0	12.0	4.25	8.00
M	BMV1+p	MT20	4.0	9.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT VERT				
M	10835	0	10835	0
G	10186	0	10186	0

UNFACTORED REACTIONS

	1ST CASE	MAX / MIN	COMPONENT REACTIONS
JT COMBINED			
M	8673	4910 / 0	1913 / 0
G	8313	4710 / 0	1831 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. FURLIN SPACING = 4.61 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x6 DRY SPF No.2 T-BRACE AT F-G, C-I, D-H

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO			FR-TO		
M-A	-8409 / 0	0.0	0.0 0.36 (1)	6.28	A-L	0 / 12883	0.89 (1)
A-B	-5935 / 0	-84.3	-84.3 0.11 (1)	4.61	L-B	-230 / 59	0.01 (1)
B-C	-9938 / 0	-84.3	-84.3 0.10 (1)	4.61	L-C	-5429 / 0	0.54 (1)
C-D	-8410 / 0	-84.3	-84.3 0.07 (1)	4.96	K-C	0 / 3819	0.18 (1)
D-E	-2001 / 0	-84.3	-84.3 0.04 (1)	6.25	C-I	-8602 / 0	0.42 (1)
E-F	-1852 / 0	-84.3	-84.3 0.02 (1)	6.25	I-D	0 / 10455	0.56 (1)
F-G	-9893 / 0	0.0	0.0 0.97 (1)	7.81	D-H	-9135 / 0	0.71 (1)
					H-E	0 / 820	0.04 (1)
					H-F	0 / 10018	0.54 (1)
M-N	0 / 0	-28.0	-28.0 0.20 (1)	10.00			
N-O	0 / 0	-28.0	-28.0 0.20 (1)	10.00			
O-L	0 / 0	-28.0	-28.0 0.20 (1)	10.00			
L-P	0 / 14064	-28.0	-28.0 0.36 (1)	10.00			
P-K	0 / 14064	-28.0	-28.0 0.36 (1)	10.00			
K-Q	0 / 13912	-28.0	-28.0 0.47 (1)	10.00			
Q-J	0 / 13912	-28.0	-28.0 0.47 (1)	10.00			
J-R	0 / 13912	-28.0	-28.0 0.47 (1)	10.00			
R-S	0 / 13912	-28.0	-28.0 0.47 (1)	10.00			
S-I	0 / 13912	-28.0	-28.0 0.47 (1)	10.00			
I-T	0 / 7029	-28.0	-28.0 0.37 (1)	10.00			
T-U	0 / 7029	-28.0	-28.0 0.37 (1)	10.00			
U-H	0 / 7029	-28.0	-28.0 0.37 (1)	10.00			
H-V	0 / 0	-28.0	-28.0 0.04 (1)	10.00			
V-G	0 / 0	-28.0	-28.0 0.04 (1)	10.00			

REGISTERED PROFESSIONAL ENGINEER

12/1/13

S. KATSOUidakOS

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.
L	4-5-12	-1866	-1866	---	BACK VERT
N	5-12	-1874	-1874	---	BACK VERT
O	2-5-12	-1866	-1866	---	BACK VERT
P	6-5-12	-1866	-1866	---	BACK VERT

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 240 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.64")
CALCULATED VERT. DEFL.(LL) = L/999 (0.16")
ALLOWABLE DEFL.(TL) = L/360 (0.64")
CALCULATED VERT. DEFL.(TL) = L/921 (0.25")

CSI: TC=0.97/1.00 (F-G:1), BC=0.47/1.00 (I-K:1), WB=0.71/1.00 (D-H:1), SS=0.63/1.00 (H-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

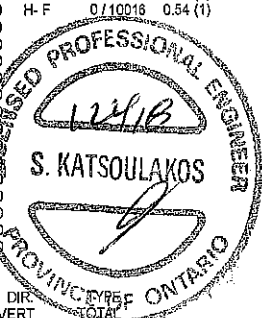
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1687 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.98 (J) (INPUT = 1.00)



DWG NO. TAM 5064-18
STRUCTURAL
COMPONENT ONLY

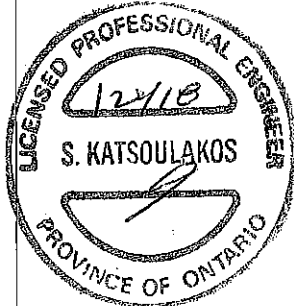
JOB NAME 286141	TRUSS NAME T20	QUANTITY 1	PLY 3	JOB DESC. TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington Version 8.200 S Dec 12 2017 MiTek Industries, Inc. Tue Jan 23 09:13:46 2018 Page 2 ID:LJPBf7XjyxdQBIYCH4AXRGyehXO-9UgllMUPjxfU7FuAHhQ_QJOaEJRr68lO_6oJ3zsXPZ					

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S)
 REQUIRED TO SUPPORT CONCENTRATED
 LOAD(S) 1873.6 lbs FACTORED DOWN AT 5-12,
 1866.3 lbs FACTORED DOWN AT 2-5-12, 1866.3
 lbs FACTORED DOWN AT 4-5-12, 1866.3 lbs
 FACTORED DOWN AT 6-5-12, 1866.3 lbs
 FACTORED DOWN AT 8-5-12, 1866.3 lbs
 FACTORED DOWN AT 10-5-12, 1866.3 lbs
 FACTORED DOWN AT 12-5-12, 1866.3 lbs
 FACTORED DOWN AT 14-5-12, AND 1866.3 lbs
 FACTORED DOWN AT 16-5-12, AND 1866.3 lbs
 FACTORED DOWN AT 17-11-12 ON BOTTOM
 CHORD. DESIGN FOR UNSPECIFIED
 CONNECTION(S) IS DELEGATED TO THE
 BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
Q	8-5-12	-1866	-1866	—	BACK	VERT	TOTAL
R	10-5-12	-1866	-1866	—	BACK	VERT	TOTAL
S	12-5-12	-1866	-1866	—	BACK	VERT	TOTAL
T	14-5-12	-1866	-1866	—	BACK	VERT	TOTAL
U	16-5-12	-1866	-1866	—	BACK	VERT	TOTAL
V	17-11-12	-1866	-1866	—	BACK	VERT	TOTAL



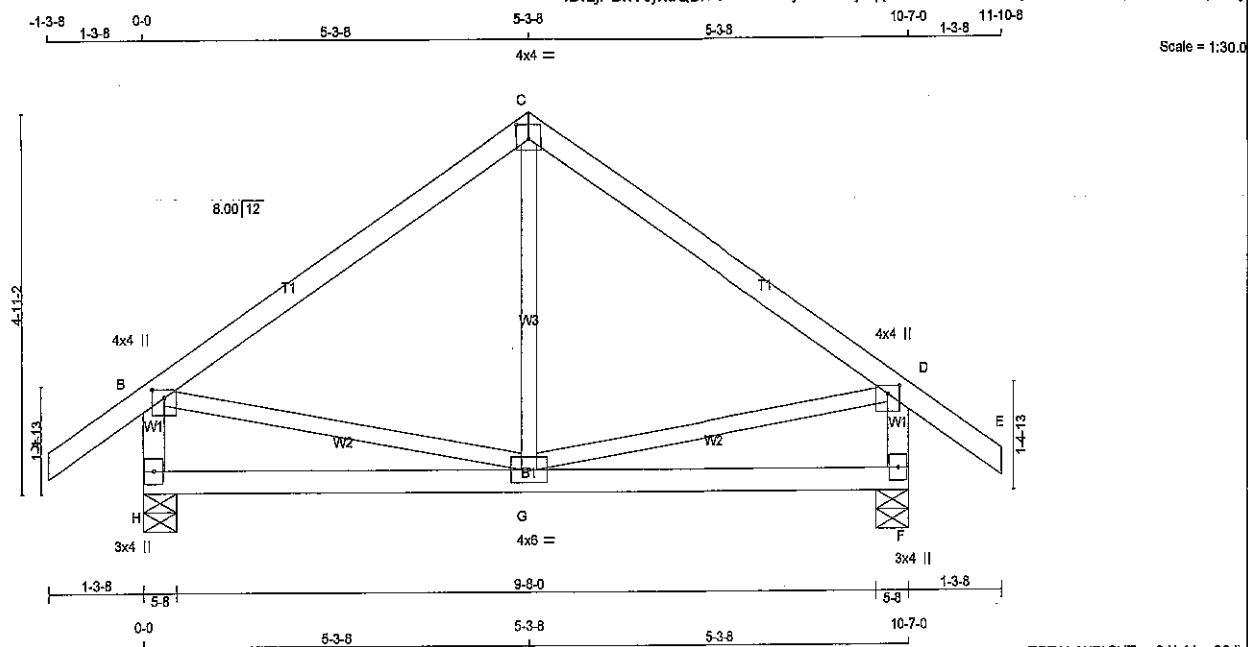
DWG NO. TAM 5064-18
 STRUCTURAL
 COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
286139	T21	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 M/Tek Industries, Inc. Mon Sep 11 16:32:01 2017 Page 1

ID: LJPBf7XlyxdQBiyCH4AXRGyehXO-Jj5qpxi84ZIOH?SaSdojVRKArPUw9pkNG7Xnbyefky



Scale = 1:30.0

TOTAL WEIGHT = 2 X 44 = 88 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	DRY	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
H - B	2x4	DRY	No.2
F - D	2x4	DRY	No.2
H - F	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMW+p	MT20	4.0	4.0	1.25	2.00
C TTW-p	MT20	4.0	4.0	2.25	2.00
D TMW+p	MT20	4.0	4.0	1.25	2.00
F BMV1+p	MT20	3.0	4.0		
G BMWW-t	MT20	4.0	6.0		
H BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
H	710	0	710	0	5-8	5-8
F	710	0	710	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
H	566	341 / 0	111 / 0	0 / 0	0 / 0	114 / 0	0 / 0
F	566	341 / 0	111 / 0	0 / 0	0 / 0	114 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 MAX (PLF)		MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED MAX. UNBRACED LENGTH FR-TO (LC)	
		FROM	TO			FR-TO	LC
A-B	0 / 32	-84.3	-84.3	G-C	0 / 238	0.05	(3)
B-C	-434 / 0	-84.3	-84.3	B-G	0 / 368	0.08	(1)
C-D	-434 / 0	-84.3	-84.3	G-D	0 / 368	0.08	(1)
D-E	0 / 32	-84.3	-84.3				
H-B	-653 / 0	0.0	0.0				
F-D	-653 / 0	0.0	0.0				
H-G	0 / 0	-28.0	-28.0				
G-F	0 / 0	-28.0	-28.0				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	25.6	PSF
DL	=	3.0	PSF
BOT CH. LL	=	10.5	PSF
DL	=	7.0	PSF
TOTAL LOAD	=	48.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.35")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.02")
ALLOWABLE DEFL.(TL) = $L/360$ (0.35")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.04")

CSI: TC=0.30 (C-D:1), BC=0.23 (G-H:3), WB=0.08 (B-G:1), SSI=0.14 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

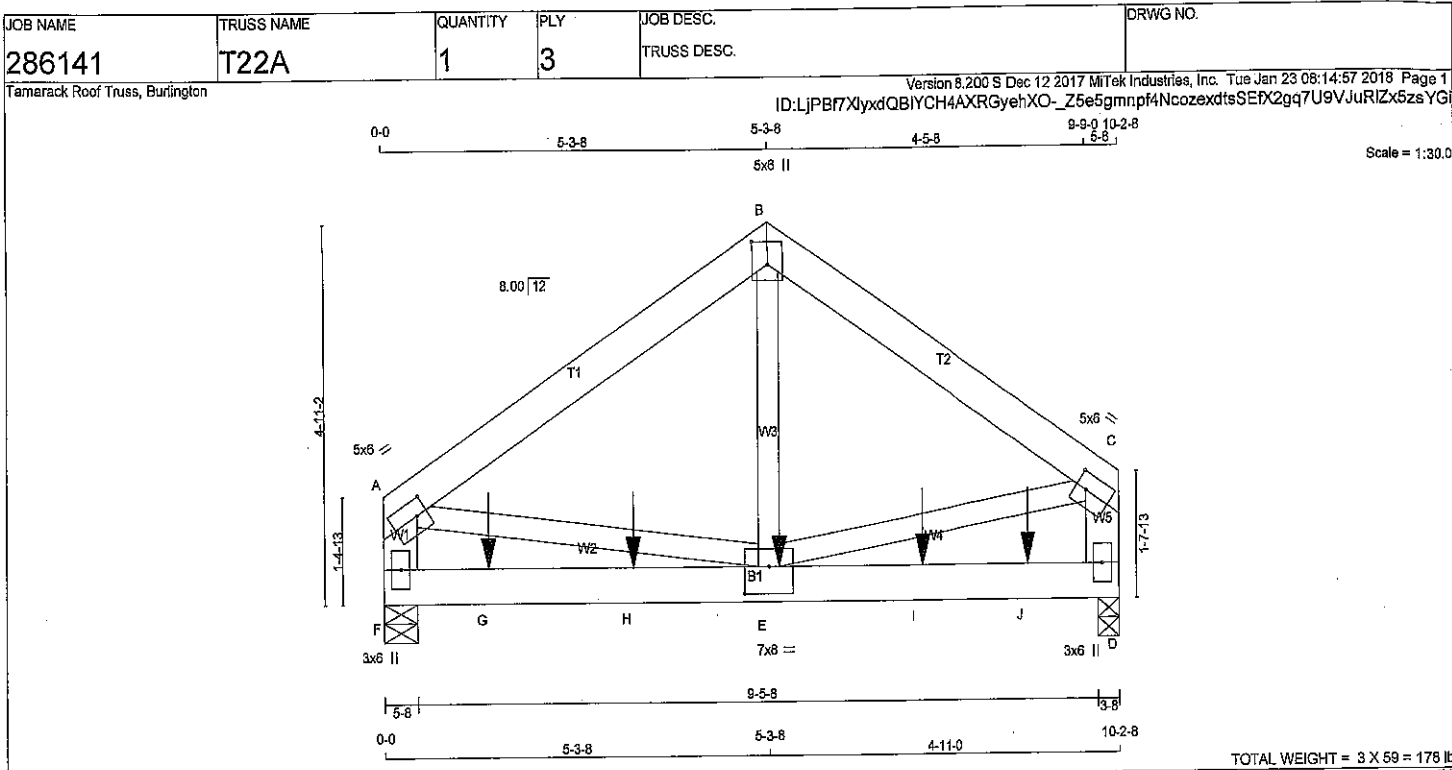
JSI GRIP= 0.51 (G) (INPUT = 0.90)
JSI METAL= 0.15 (B) (INPUT = 1.00)



HWONO, YAM 46167-17
STRUCTURAL
COMPONENT ONLY



DWG NO. TAM 46173-17
STRUCTURAL
COMPONENT ONLY



LUMBER				
N. L. G. A. RULES	SIZE	LUMBER	DESCR.	
CHORDS				
A - B	2x6 DRY	No.2	SPF	
B - C	2x6 DRY	No.2	SPF	
F - A	2x6 DRY	No.2	SPF	
D - C	2x6 DRY	No.2	SPF	
F - D	2x6 DRY	No.2	SPF	
ALL WEBS EXCEPT	2x4 DRY	No.2	SPF	

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
A - B	2 12	TOP
B - C	2 12	TOP
F - A	2 12	TOP
D - C	2 12	TOP
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
F - D	2 7	SIDE (973.7)
WEBS: (0.122"x3") SPIRAL NAILS		
2x4	1 6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
A	TM/VW-t	MT20	5.0	6.0	2.50	1.75
B	TTW+p	MT20	5.0	6.0	3.50	2.50
C	TM/VW-t	MT20	5.0	6.0	2.50	1.75
D	BMV1+p	MT20	3.0	6.0		
E	BMVWW-t	MT20	7.0	8.0	4.25	4.00
F	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1880.6 lbs FACTORED DOWN AT 1-5-4, 1866.3 lbs FACTORED DOWN AT 3-5-4, 1866.3 lbs FACTORED DOWN AT 5-5-4, AND 1866.3 lbs FACTORED DOWN AT 7-5-4, AND 1866.3 lbs FACTORED DOWN AT 8-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
F	5150	0	5150	0	5-8
D	5442	0	5442	0	3-8

UNFACTORED REACTIONS		1ST CASE	MAX/MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
F	4186	2393 / 0	807 / 0
D	4434	2516 / 0	974 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.15 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS					WEBS			
MAX. FACTORED		FACTORED			MAX. FACTORED			
MEMB.	FORCE	VERT. LOAD	LC1	MAX	MEMB.	FORCE	MAX	
	(LBS)	(PLF)		CSI (LC)		(LBS)	CSI (LC)	
FR-TO		FROM	TO	LENGTH	FR-TO			
A-B	-4750 / 0	-84.3	-84.3	0.08 (1)	6.15	E-B	0 / 4840 0.26 (1)	
B-C	-4750 / 0	-84.3	-84.3	0.07 (1)	6.16	A-E	0 / 4009 0.21 (1)	
F-A	-3527 / 0	0.0	0.0	0.08 (1)	7.81	E-C	0 / 4058 0.22 (1)	
D-C	-3764 / 0	0.0	0.0	0.08 (1)	7.81			
F-G	0 / 0	-28.0	-28.0	0.65 (1)	10.00			
G-H	0 / 0	-28.0	-28.0	0.65 (1)	10.00			
H-E	0 / 0	-28.0	-28.0	0.65 (1)	10.00			
E-I	0 / 0	-28.0	-28.0	0.65 (1)	10.00			
I-J	0 / 0	-28.0	-28.0	0.65 (1)	10.00			
J-D	0 / 0	-28.0	-28.0	0.65 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)		LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE
JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	
E	5-5-4	-1866	-1866	---	FRONT	VERT	TOTAL	
G	1-5-4	-1881	-1881	---	FRONT	VERT	TOTAL	
H	3-5-4	-1866	-1866	---	FRONT	VERT	TOTAL	
I	7-5-4	-1866	-1866	---	FRONT	VERT	TOTAL	
J	8-11-4	-1866	-1866	---	FRONT	VERT	TOTAL	

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL) = L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/999 (0.10")

CSI: TC=0.08/1.00 (C-D:1), BC=0.65/1.00 (E-F:1), WB=0.26/1.00 (B-E:1), SS=0.48/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.41 (A) (INPUT = 1.00)



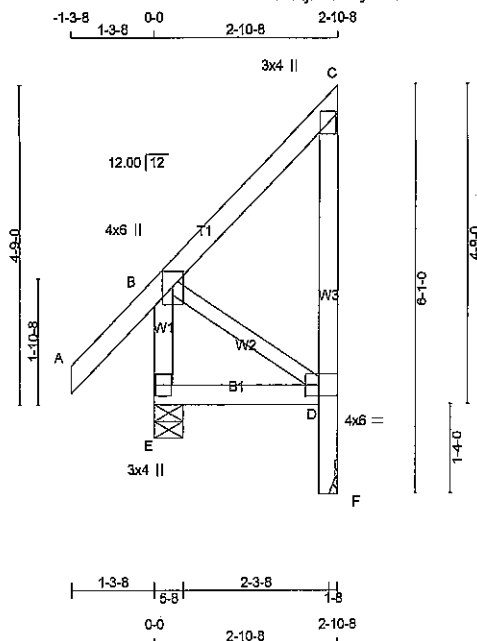
DWG NO. TAM 5065
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43994	DRWG NO.
286139	T23	8	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID: LJPB77XlyxdQBIYCH4AXRGyehXO-nvfC49yxvOn90Rae8991FjzYqFnCfdLtcwt5J1yefkx



Scale = 1/32

TOTAL WEIGHT = 8 X 20 = 163 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
F - C	2x4	DRY	No.2	SPF
E - B	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C	TMV+p	MT20	3.0	4.0		
D	BVMV+l	MT20	4.0	6.0	2.00	2.50
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
F	161	0	161	0	5-8	5-8
E	279	0	279	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	132	74 / 0	30 / 0	0 / 0	0 / 0	29 / 0	0 / 0
E	212	145 / 0	30 / 0	0 / 0	0 / 0	37 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS		WEBS			
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRACED LENGTH	MEMB. FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO						
A-B	0 / 42	-84.3	-84.3	0.12 (1)	10.00	0 / 0
B-C	0 / 0	-84.3	-84.3	0.12 (1)	10.00	0 / 0
F-D	-161 / 0	0.0	0.0	0.02 (1)	7.81	0 / 0
D-C	-121 / 0	0.0	0.0	0.05 (1)	7.81	0 / 0
E-B	-239 / 0	0.0	0.0	0.03 (1)	7.81	0 / 0
E-D	0 / 0	-28.0	-28.0	0.07 (3)	10.00	0 / 0

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL)= L/999 (0.01")

CSI: TC=0.12 (B-C:1), BC=0.07 (D-E:3), WB=0.00 (B-D:1), SSI=0.07 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1567
	822	2284	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)

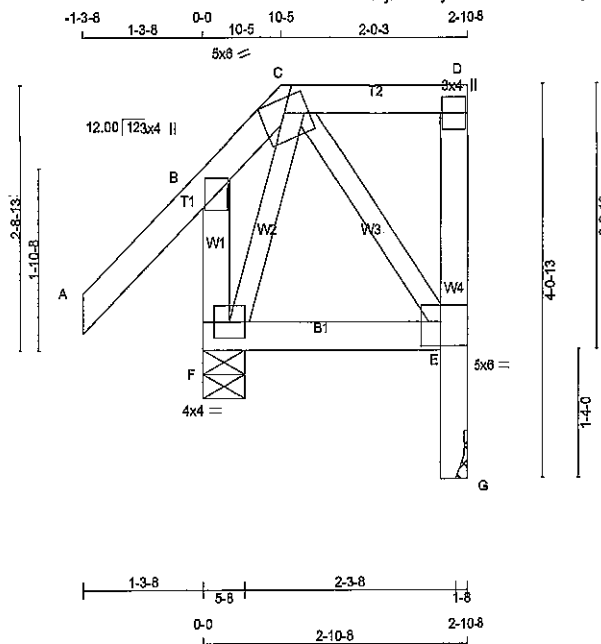


DWG NO. TAN 46169-17
STRUCTURAL
COMPONENT ONLY

JOB NAME 286139	TRUSS NAME T24	QUANTITY 2	PLY 1	JOB DESC. 43954 TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale: 1/2"=1'

TOTAL WEIGHT = 2 X 19 = 38 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	DESCR.
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
G - D	2x4	DRY	No.2
F - B	2x4	DRY	No.2
F - E	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)				
JT. TYPE	PLATES	W	LEN	Y X
B TMV+p	MT20	3.0	4.0	
C TTWW-m	MT20	5.0	6.0	Edge
D TMV+p	MT20	3.0	4.0	
E BVWW-I	MT20	5.0	6.0	3.00 2.50
F BVWW-I	MT20	4.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
G	133	0	133	0	0	HANGER BY OTHERS
F	307	0	307	0	0	MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. SNOW	MAX/MIN. LIVE	MAX/MIN. PERM. LIVE	MAX/MIN. WIND	MAX/MIN. DEAD	MAX/MIN. SOIL
G	113	56/-17	30/0	0/0	0/0	27/0	0/0
F	232	162/0	30/0	0/0	0/0	39/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 MAX. (CSI (LC) UNBRACED LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 MAX. (CSI (LC) UNBRACED LENGTH FR-TO
A-B	0/42	-84.3	-84.3 0.12 (1)	10.00	F-C	-24/58	0.01 (5)
B-C	-62/0	-84.3	-84.3 0.11 (1)	6.25	C-E	-14/33	0.01 (5)
C-D	0/0	-84.3	-84.3 0.05 (1)	10.00			
G-E	-133/0	0.0	0.0 0.01 (1)	7.81			
E-D	-82/0	0.0	0.0 0.01 (1)	7.81			
F-B	-244/0	0.0	0.0 0.03 (1)	7.81			
F-E	-21/8	-28.0	-28.0 0.07 (3)	6.25			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 3.0 PSF
BOT CH. LL	= 10.5 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.12 (A-B:1), BC=0.07 (E-F:3), WB=0.01 (C-F:5), SSI=0.07 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 616 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (F) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)



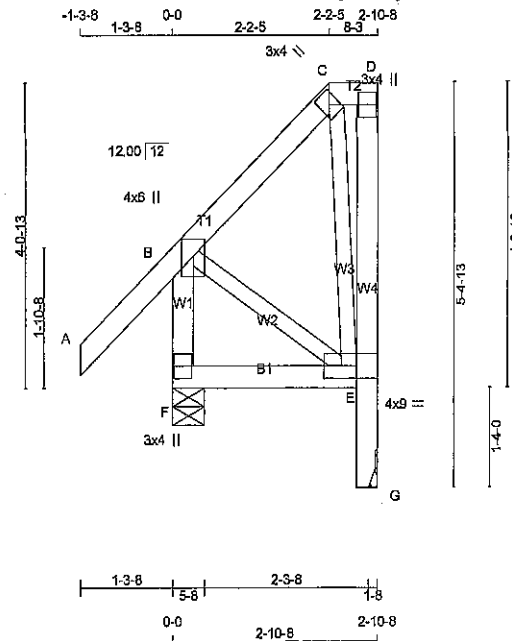
DWG NO. TAN46170-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
286139	T25	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1:30.9

TOTAL WEIGHT = 2 X 22 = 44 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
G - D	2x4	DRY	No.2	SPF
F - B	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C	TTW+h	MT20	3.0	4.0		
D	TMV+p	MT20	3.0	4.0		
E	BVMW+h	MT20	4.0	9.0	2.00	3.00
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	HORZ	DOWN	UP
G	161	0	161	0
F	279	0	279	0

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
G	132	74 / 0	30 / 0	0 / 0	0 / 0	29 / 0	0 / 0	0 / 0
F	212	145 / 0	30 / 0	0 / 0	0 / 0	37 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRACED LENGTH	WEBS	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH
FR-TO					FR-TO		
A-B	0 / 42	-84.3	-84.3 0.12 (1)	10.00	C-E	-104 / 0	0.03 (1)
B-C	-27 / 0	-84.3	-84.3 0.07 (1)	6.25	B-E	0 / 21	0.00 (1)
C-D	0 / 0	-84.3	-84.3 0.01 (1)	10.00			
G-E	-161 / 0	0.0	0.0 0.02 (1)	7.81			
E-D	-29 / 0	0.0	0.0 0.01 (1)	7.81			
F-B	-239 / 0	0.0	0.0 0.03 (1)	7.81			
F-E	0 / 0	-28.0	-28.0 0.07 (3)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 3.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = 1/998 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.01")

CSI: TC=0.12 (A-B:1), BC=0.07 (E-F:3), WB=0.03 (C-E:1), SSI=0.06 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

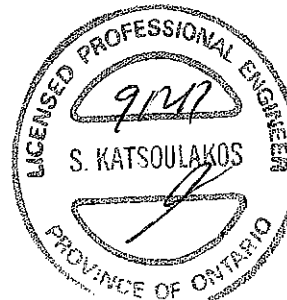
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX MIN
MT20	618	354	1987 822 2284 1658

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.80)
JSI METAL= 0.04 (C) (INPUT = 1.00)



DRWG NO. TAM 4671 -17
STRUCTURAL
COMPONENT ONLY

LUL/LUS/LJS/HUS/HHUS/HGUS

Standard and Double-Shear Joist Hangers



This product is preferable to similar connectors because of
a) easier installation, b) higher capacities, c) lower installed
cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 258–259.

Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 20–24.

Installation:

- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on p. 126.



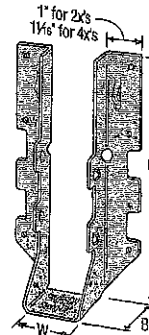
Double-Shear
Nailing
Top View



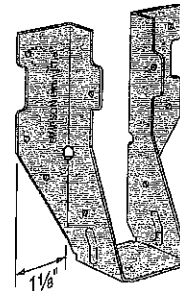
Double-Shear
Nailing
Side View;
Do not
bend tab



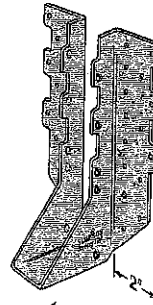
Dome Double-Shear
Nailing Side View
(available on
some models)
U.S. Patent 5,603,580



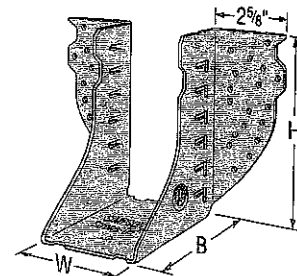
✓ LUS28



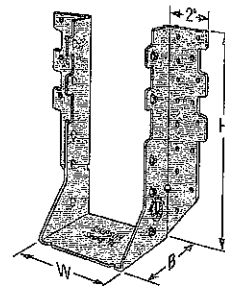
LU26L



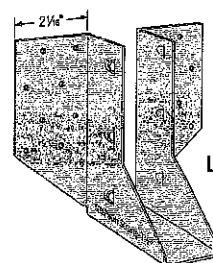
✓ HUS210
(HUS26, HUS28,
and HHUS similar)



✓ HGUS28-2

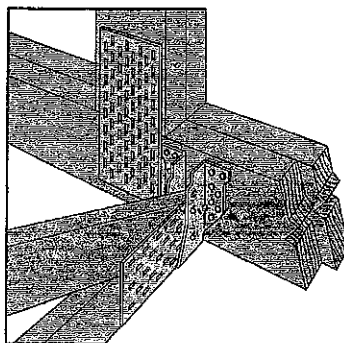


✓ HHUS210-2



LJS26DS

**Typical HUS26
Installation
with Reduced
Heel Height**
(Truss Designer
to provide
fastener quantity
for connecting
multiple members
together)



Plated Truss Connectors

LUL/LUS/LJS/HUS/HHUS/HGUS

HHUS/HGUS

See Hanger Options information on pp. 125–127.

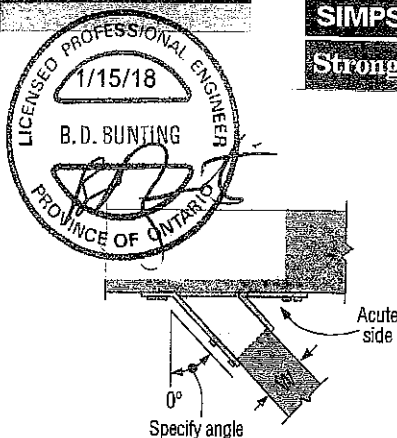
HHUS — Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.62 of the table value
- The joist must be bevel-cut to allow for double-shear nailing

HGUS — Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Joist	Down Resistance	Uplift
W < 2"	Bevel or square cut	0.62 of table value	0.46 of table value
2" < W < 6"	Bevel cut	0.67 of table value	0.41 of table value
2" < W < 6"	Square cut	0.46 of table value	0.41 of table value
W > 6"	Bevel cut	0.75 of table value	0.41 of table value



**Top View HHUS Hanger
Skewed Right**
(Joist must be bevel cut)
All joist nails installed on the
outside angle (non-acute side).

Standard and Double-Shear Joist Hangers (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance			
		W	H	B	d _e ³	Header	Joist	D-Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
								(K _D = 1.15)	(K _D = 1.00)	(K _D = 1.15)	(K _D = 1.00)
lb.	lb.	lb.	lb.								
kN	kN	kN	kN								
Single 2x Sizes											
LUS24	18	1⅞	3⅝	1⅜	2¼	(4) 10d	(2) 10d	710	1625	645	1155
								3.16	7.23	2.87	5.14
LU24L	22	1⅞	3	1⅞	2⅞	(4) 10d	(2) 10d x 1½"	360	1020	320	725
								1.60	4.54	1.42	3.22
LU26L	22	1⅞	5	1⅞	4⅝	(6) 10d	(4) 10d x 1½"	720	1605	645	1140
								3.20	7.14	2.87	5.07
LUS26	18	1⅞	4¾	1⅜	3¾	(4) 10d	(4) 10d	1420	2170	1290	1630
								6.32	9.65	5.74	7.25
HUS26	16	1⅞	5¾	3	3⅞	(14) 16d	(6) 16d	2705	4940	2065	3875
								11.30	21.97	9.20	17.24
LJS26DS	18	1⅞	5	3½	4⅝	(16) 16d	(6) 16d	2055	4265	1460	4115
								9.14	18.97	6.49	18.31
HGUS26	12	1⅞	5¾	5	4⅞	(20) 16d	(8) 16d	2685	6625	2685	5700
								11.96	29.51	11.96	25.35
LU26L	20	1⅞	6¾	1⅞	5⅞	(8) 10d	(6) 10d x 1½"	1140	2185	1020	1550
								5.07	9.72	4.54	6.89
LUS28	18	1⅞	6⅞	1⅜	3¾	(6) 10d	(4) 10d	1420	2520	1290	1790
								6.32	11.21	5.74	7.96
HUS28	16	1⅞	7⅞	3	6⅞	(22) 16d	(8) 16d	3605	5365	2675	4345
								16.04	23.86	11.90	19.33
HGUS28	12	1⅞	7⅞	5	6⅞	(36) 16d	(12) 16d	3310	7675	3310	6900
								14.74	34.19	14.74	30.73
LU210L	20	1⅞	8	1⅞	7⅞	(10) 10d	(6) 10d x 1½"	1140	2495	1020	1770
								5.07	11.10	4.54	7.87
LUS210	18	1⅞	7⅞	1⅜	3¾	(8) 10d	(4) 10d	1420	2785	1290	2210
								6.32	12.39	5.74	9.83

1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.
3. d_e is the distance from the bearing seat to the top joist nail.
4. Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-C-N10TRSSCN and/or see installation notes.
5. Nails: 16d = 0.162" dia. x 3 1/4" long. See pp. 27–28 for other nail sizes and information.

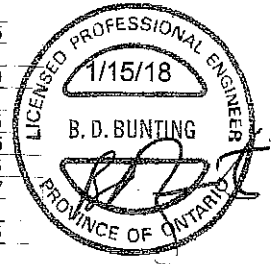
Face-Mount Hangers

SIMPSON
Strong-Tie

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

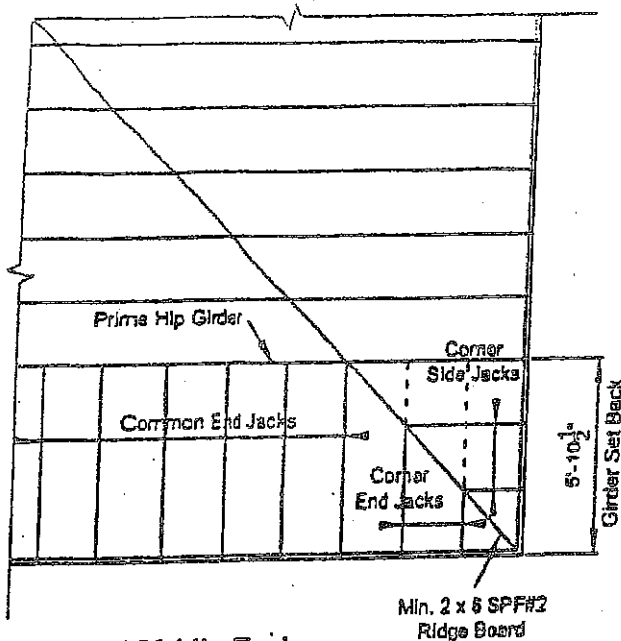
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance			
		W	H	B	d _g ³	Header	Joist	D-Flr-L		S-P-F	
								Uplift	Normal	Uplift	Normal
								(K _D = 1.15)	(K _D = 1.00)	(K _D = 1.15)	(K _D = 1.00)
								lb.	lb.	lb.	lb.
								kN	kN	kN	kN
Double 2x Sizes											
LUS24-2	18	3 1/8	3 1/8	2	1 1/2	(4) 16d	(2) 16d	835	2020	590	1435
								3.71	8.99	2.62	6.38
LUS26-2	18	3 1/8	4 1/8	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
								7.65	11.54	6.87	8.54
HHUS26-2	14	3 1/8	5 1/8	3	3 1/8	(14) 16d	(8) 16d	2850	7335	2065	5205
								12.68	32.63	9.20	23.15
HGUS26-2	12	3 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
								7.65	14.79	6.87	11.45
HHUS28-2	14	3 1/8	7 1/8	3	6 1/8	(22) 16d	(8) 16d	3765	8940	2675	6345
								16.75	39.77	11.90	28.22
HGUS28-2	12	3 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
								11.48	20.02	10.32	14.21
HHUS210-2	14	3 1/8	9 1/8	3	8	(30) 16d	(10) 16d	4670	9660	4235	7000
								20.77	42.97	18.84	31.14
HGUS210-2	12	3 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	6840	14015	4855	10270
								30.43	62.34	21.60	45.69
Triple 2x Sizes											
HGUS26-3	12	4 1/8	5 1/2	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
HGUS28-3	12	4 1/8	7 1/4	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
HHUS210-3	14	4 1/8	9	3	7 1/8	(30) 16d	(10) 16d	4670	9670	4235	6865
								20.77	43.02	18.84	30.54
HGUS210-3	12	4 1/8	9 1/4	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400
								30.43	65.14	21.60	46.26
Quadruple 2x Sizes											
HGUS26-4	12	6 1/8	5 7/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
HGUS28-4	12	6 1/8	7 3/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
HHUS210-4	14	6 1/8	8 1/8	3	7 1/8	(30) 16d	(10) 16d	4670	10155	4235	7210
								20.77	45.17	18.84	32.07
HGUS210-4	12	6 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400
								30.43	65.14	21.60	46.26
HGUS212-4	12	6 1/8	10 1/8	4	10 1/8	(56) 16d	(20) 16d	7640	14995	5425	10645
								33.98	66.70	24.13	47.35
HGUS214-4	12	6 1/8	12 1/8	4	11 1/8	(66) 16d	(22) 16d	10130	16400	7195	11645
								45.06	72.95	32.00	51.80
4x Sizes											
LUS46	18	3 1/8	4 1/8	2	3 1/8	(4) 16d	(4) 16d	1720	2595	1545	1920
								7.65	11.54	6.87	8.54
HHUS46	14	3 1/8	5 1/8	3	3 1/8	(14) 16d	(8) 16d	2850	7335	2065	5205
								11.30	32.63	9.20	23.15
HGUS46	12	3 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
LUS48	18	3 1/8	6 1/8	2	3 1/8	(6) 16d	(4) 16d	1720	3325	1545	2575
								7.65	14.79	6.87	11.45
HHUS48	14	3 1/8	7 1/8	3	6 1/8	(22) 16d	(8) 16d	3765	8940	2675	6345
								16.75	39.77	11.90	28.22
HGUS48	12	3 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
LUS410	18	3 1/8	8 1/8	2	5 1/8	(8) 16d	(8) 16d	2580	4500	2320	3195
								11.48	20.02	10.32	14.21
HGUS410	12	3 1/8	9	4	8 1/8	(46) 16d	(16) 16d	6840	14015	4855	10270
								30.43	62.34	21.60	45.69
HGUS412	12	3 1/8	10 1/8	4	10 1/8	(56) 16d	(20) 16d	7640	14995	5425	10645
								33.98	66.70	24.13	47.35
HGUS414	12	3 1/8	12 1/8	4	11 1/8	(66) 16d	(22) 16d	10130	16400	7195	11645
								45.06	72.95	32.00	51.80



Plated Truss Connectors

MICRO CITY ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242



45° Hip End

LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2

BOTTOM CHORD : 2 x 4 SPF#2

WEBS : 2 x 3 SPF#2

UNLESS OTHERWISE SHOWN

DESIGN LOAD:

TOP CHORD LIVE LOAD : 34.8 P.S.F.

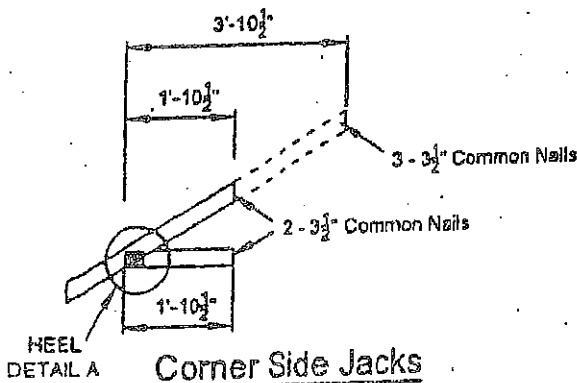
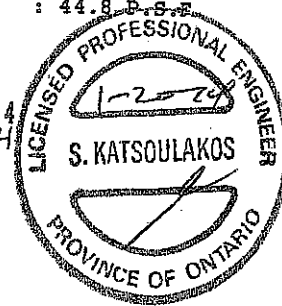
TOP CHORD DEAD LOAD : 3.0 P.S.F.

BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.

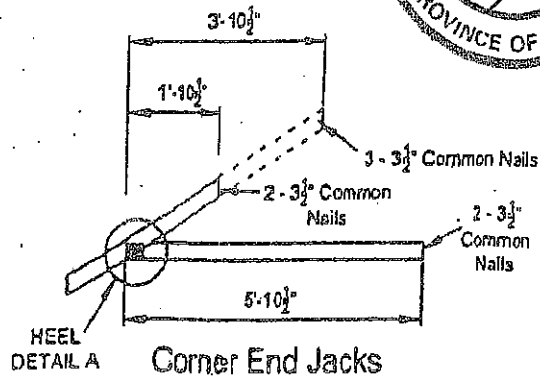
BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD : 44.8 P.S.F.

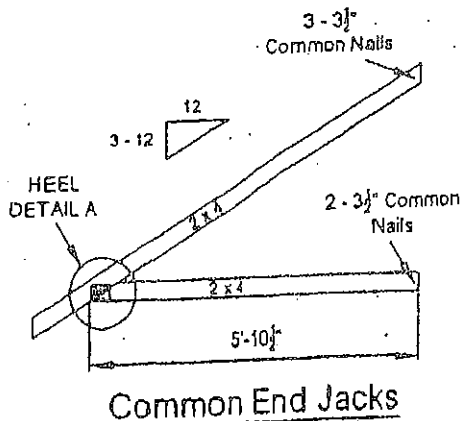
DWG NO TAM 3495.14
STRUCTURAL
COMPONENT ONLY



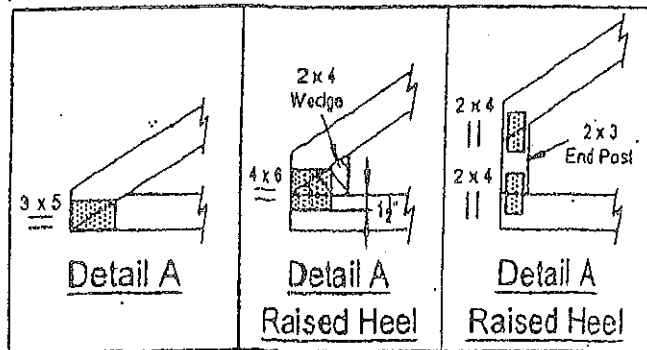
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)

(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

MICRO CITY ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

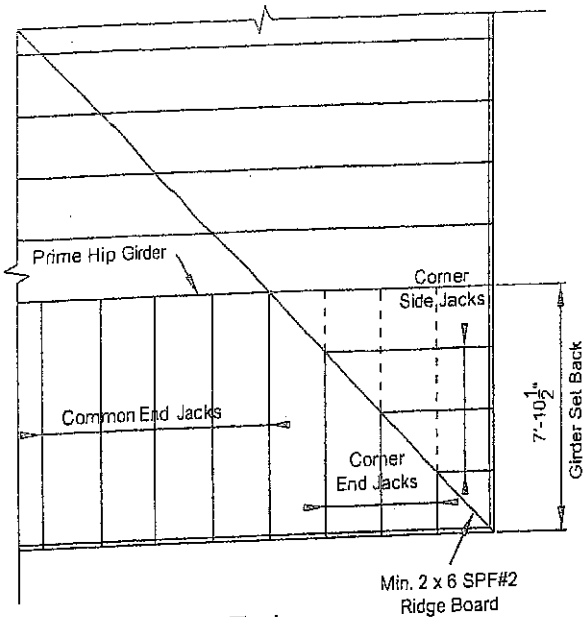
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

DESIGN LOAD:

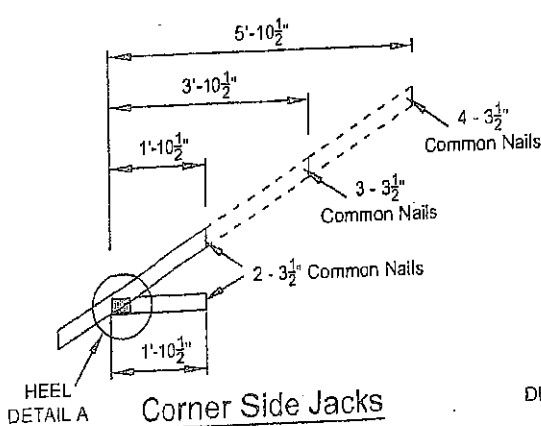
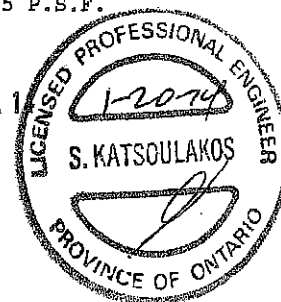
TOP CHORD LIVE LOAD : 40.5 P.S.F.
TOP CHORD DEAD LOAD : 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD : 50.5 P.S.F.

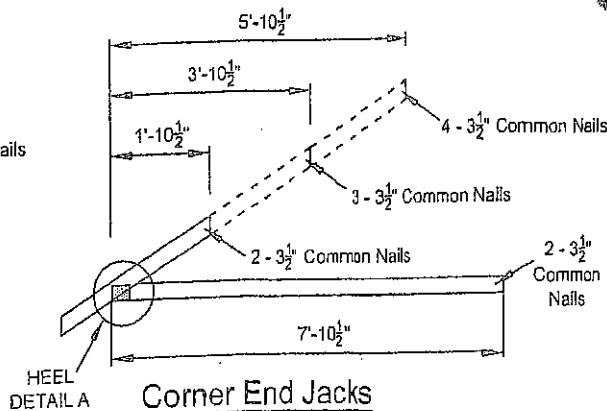


45° Hip End

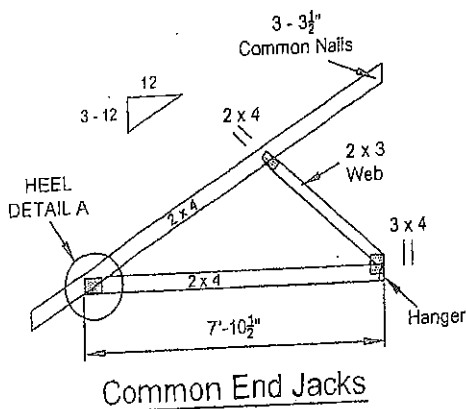
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STRUCTURAL
COMPONENT ONLY



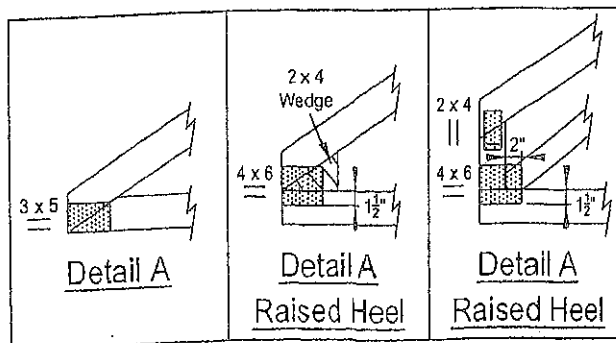
Corner Side Jacks



Corner End Jacks



Common End Jacks



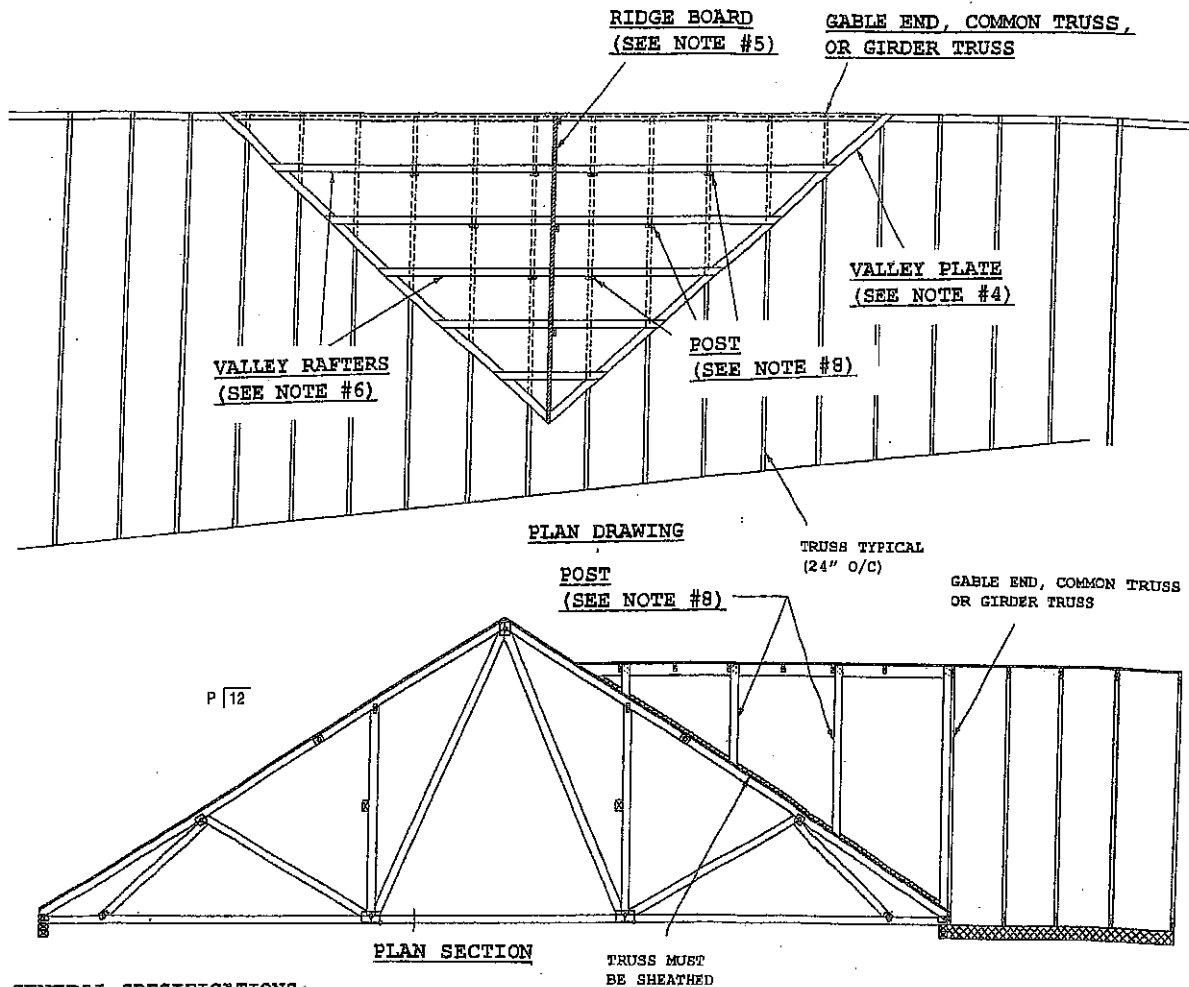
NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

MICRO CITY ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

CONVENTIONAL VALLEY FRAMING DETAIL



GENERAL SPECIFICATIONS:

- (1) WITH THE BASE TRUSSES ERECTED (INSTALLED), APPLY SHEATHING TOP CHORD OF SUPPORTING (BASE) TRUSSES.
- (2) BRACE BOTTOM CHORD AND WEB MEMBERS AS PER PRE-ENGINEERED TRUSS DESIGNS.
- (3) DEFINE VALLEY RIDGE BY RUNNING A LEVEL STRING FROM THE INTERSECTING RIDGE OF THE (a) GABLE END, (b) GIRDER TRUSS OR (c) COMMON TRUSS TO THE ROOF SHEATHING.
- (4) INSTALL 2 X 6 VALLEY PLATES ON FLAT. FASTEN TO EACH SUPPORTING TRUSS WITH (2) 16d (3.5" X 0.131") NAILS.
- (5) SET A 2 X 6 #2 RIDGE BOARD (MAX. 10'-0" RIDGE) OR 2 X 8 #2 SPF RIDGE BOARD (MAX. 20'-0" RIDGE). SUPPORT RIDGE BOARD WITH 2 X 4 POSTS SPACED 48" O/C. BEVEL BOTTOM OF POST TO SET EVENLY ON THE SHEATHING. FASTEN POST TO RIDGE WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST TO ROOF SHEATHING WITH (3) 10d (3" X 0.131") TOE-NAILS.
- (6) FRAME VALLEY RAFTERS FROM VALLEY PLATE TO RIDGE BOARD. MAXIMUM RAFTER SPACING IS 24" O/C. FASTEN VALLEY RAFTER TO RIDGE BEAM WITH (3) 16d (3.5" X 0.131") TOE-NAILS. FASTEN VALLEY RAFTER TO VALLEY PLATE WITH (3) 16d (3.5" X 0.131") TOE-NAILS.
- (7) SUPPORT THE VALLEY RAFTERS WITH 2 X 4 POSTS AT 48" O/C (OR LESS) ALONG EACH RAFTER. INSTALL POSTS IN A STAGGERED PATTERN AS SHOWN ON PLAN DRAWING. ALIGN POSTS WITH TRUSSES BELOW. FASTEN VALLEY RAFTER TO POST WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST THROUGH SHEATHING TO SUPPORTING TRUSSES WITH (2) 16d (3.5" X 0.131") NAILS.
- (8) POSTS SHALL BE 2 X 4 #2 SPF OR BETTER. POSTS EXCEEDING 75" IN HEIGHT SHALL BE INCREASED TO 4 X 4 #2 SPF, OR BETTER, OR BE PRE-ASSEMBLED TWO (2) PLY 2 X 4 #2 SPF OR BETTER FASTENED TOGETHER WITH 2 ROWS OF 10d (3" X 0.131") NAILS AT 6" O/C.
- (9) MAINTAIN A MINIMUM 3/4" LUMBER EDGE DISTANCE WHEN NAILING. NAIL SPACING SHOULD APPROXIMATE A MINIMUM 1-3/4" O/C OR MORE UNLESS NOTED OTHERWISE. ALL CONSTRUCTION TO CONFORM TO ONTARIO BUILDING CODE (CURRENT ADDITION) AT ALL TIMES.

NOTES:

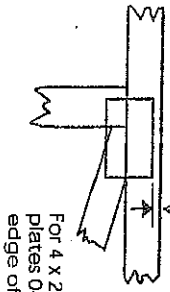
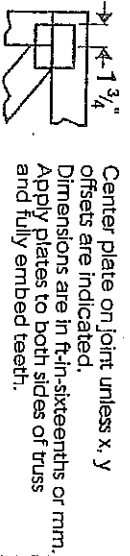
- (10) 48" O/C (MAXIMUM POST SPACING).
- (11) ROOF LIVE LOAD = 34.8 PSF (MAX.).
- (12) ROOF DEAD LOAD = 10.0 PSF (MAX.).
- (13) PART 9 APPLICATION ONLY (ONTARIO BUILDING CODE)
- (14) PART 4 APPLICATION ONLY (ONTARIO BUILDING CODE) WITH APPROVED REVIEW BY LICENSED PROFESSIONAL ENGINEER.
- (15) BASE TRUSS SPACING (24" O/C MAX.).
- (16) ALL PRE-ENGINEERED BASE TRUSS COMPONENTS TO BE SEALED BY LICENSED PROFESSIONAL ENGINEER AND THIS DETAIL TO BE VERIFIED AND APPROVED BY SAME WHEN RIDGE BOARD LENGTH EXCEEDS 12'-0".
- (17) ALL BASE TRUSSES: P = 4 (4/12) - MINIMUM.
- (18) ALL VALLEY RAFTERS: P = 4 (4/12) - MINIMUM.



DWG NO TAM 6305.14
STRUCTURAL
COMPONENT ONLY

Symbols

PLATE LOCATION AND ORIENTATION



This symbol indicates the required direction of slots in connector plates.

* Plate location details available in Mitek software or upon request.

PLATE SIZE

4 X 4

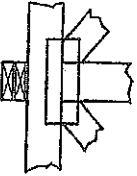
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

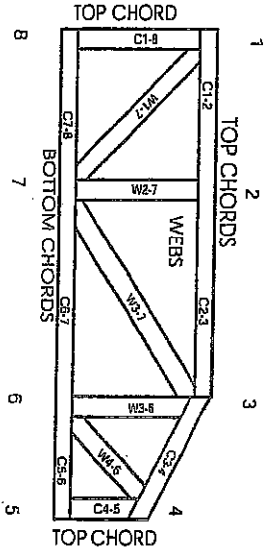
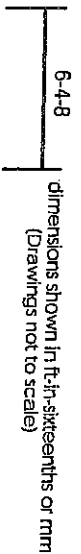
BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:
TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate
Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

T1996-L, 10319-L, 13270-L, 12691-R

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General Safety Notes

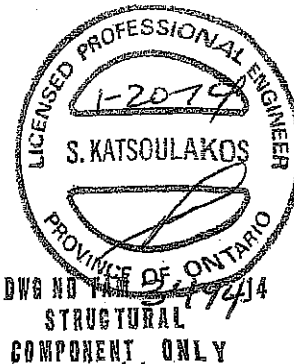
Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plying requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

Micro City Engineering Services Inc.
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N0L 1M0

(519) 287 - 2242; Fax: (519) 287 - 5750 (Call)



Responsibilities:

Micro City Engineering Services is responsible for the design of trusses as individual components.

It is the responsibilities of others to ascertain that the design loads utilized on this (these) drawing(s) meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdiction over such decisions.

All dimensions are to be verified by the owner, contractor, architect, or other authority having input over such decisions prior to truss component manufacture. At no time shall Micro City Engineering Services Inc. or its employees be responsible for dimension errors.

Micro City Engineering Services Inc. bears no responsibility for the erection of any truss components. Persons erecting truss components are cautioned to seek professional advice regarding temporary and permanent bracing systems and to be **totally** familiar with all aspects of truss erection prior to proceeding on **any** truss component erection job. Any bracing shown on Micro City Engineering Services Inc. or Tamarack Roof Trusses Inc. sealed or unsealed truss component drawings is specified for the single truss component in question and is identified as an integral part of the design for that particular truss component but is **not** meant to represent the only required bracing for that particular truss component when installed as a component in a series of truss components in a roof truss system.

It is the truss manufacturer's responsibility to ensure that trusses are manufactured in accordance with Micro City Engineering Services Inc. specifications outlined below:

SPECIFICATIONS:

Truss components sealed by Micro City Engineering Services Inc. must conform to the relevant sections of the current Building Code of Ontario and Canada (Part 4 or Part 9) or the current Farm Building Code of Canada in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the Truss Plate Institute of Canada (TPIC). All unit lumber and nailing stresses identified on truss component design drawings and/or used in the design of individual truss components shall conform to the current CSA Wood Design standard identified in the current Building Code and TPIC Design Standards.

The lumber used to manufacture any truss component is to conform to the specified size and grade identified on the truss drawing.

The lumber used in the manufacture of any truss component is not to exceed 19% during its service use unless specifically noted on the truss drawing.

The lumber used in the manufacture of any truss component is not to be treated with any chemicals during its service life unless specifically noted on the truss drawing.

Connector plates shall be applied to both faces of the truss component at each joint and shall be positioned exactly as specified.

The top chord of any truss component is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the sealed truss component drawing but not exceeding 24" o/c (Part 9 design) and not exceeding 48" o/c (Part 4 or Agricultural design).

When a truss component is to be installed with no rigid ceiling attached directly to the bottom chord, then the bottom chord is to be laterally braced at intervals not exceeding 3m (or 10'-0").

All sealed or unsealed truss component drawings provided by Micro City Engineering Services Inc. Or Tamarack Roof Trusses Inc. should be read in conjunction with the following:

Warning-Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473C rev 10-'08 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer - not the truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component Safety Information available from the Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA, 22314.