

ASPHALT SHINGLES
12" FINISH O.H.
R.T.M.C
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:

HGUS26-2 -(XX)
LJS26DS -(V)
LUS24 -(O)

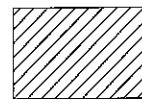
DENOTES:

H -16" RAISED PLATE

ALL B- 2-2X10 BEAMS (FLUSH)

T-170534

DENOTES:
CONVENTIONAL
FRAMING



DEWBERRY 2E
EL:1 - UNIT1

286185 D

DEWBERRY 1
EL:1 (REV)
UNIT2

286186

C OF WOOD PARTY
WALL ABOVE CEILING

DEWBERRY 1
EL:1
UNIT3

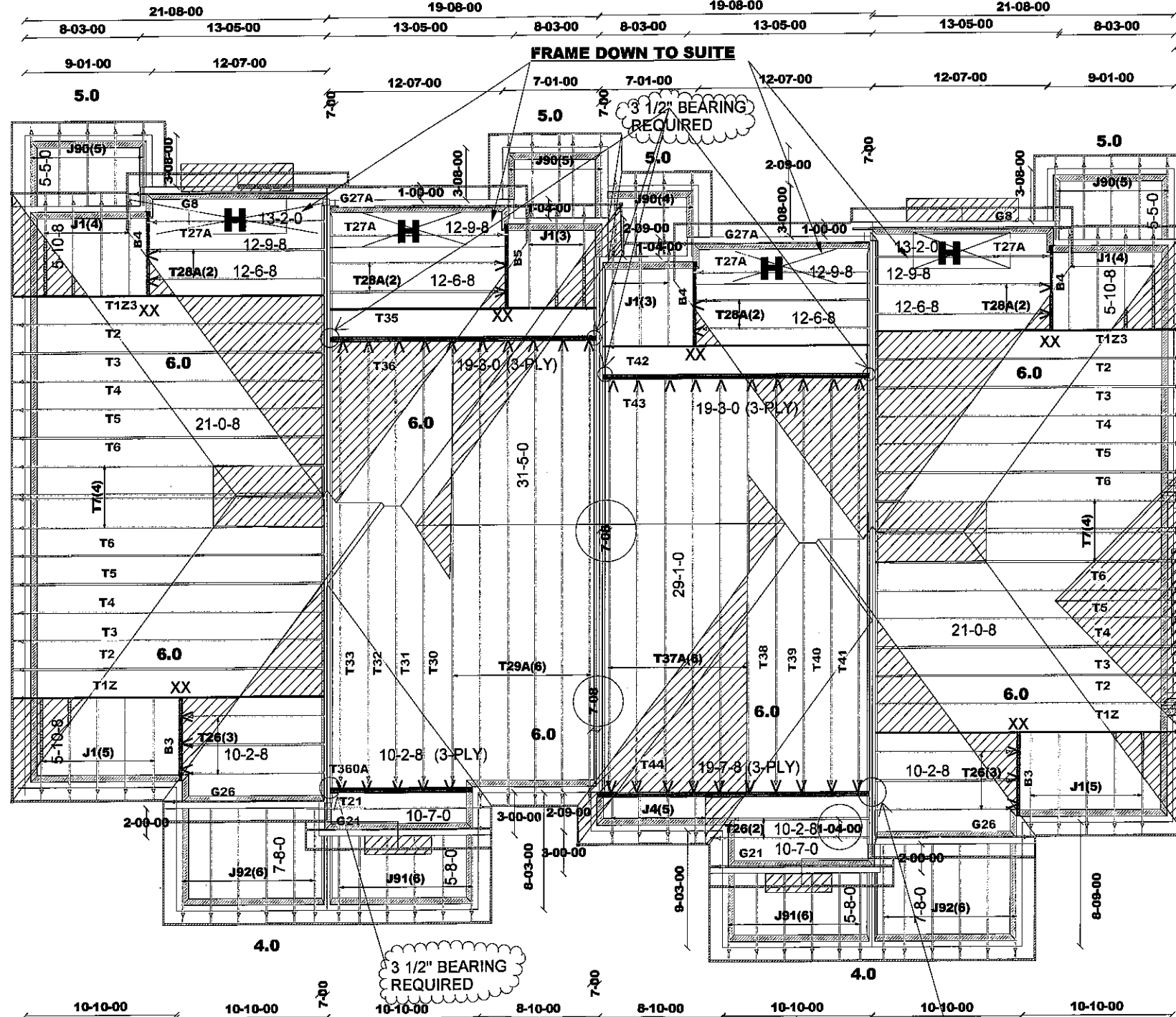
286187

C OF WOOD PARTY
WALL ABOVE CEILING

DEWBERRY 2ES
EL:1 (REV)-UNIT4

286185 D

C OF WOOD PARTY
WALL ABOVE CEILING



8/12 PITCHES (TYP.)
UNLESS NOTED

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC LATEST EDITION
ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"x4"SPF@24"o.c.
WITH A 2"x4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.
POSTS LONGER THAN 6' TO BE Laterally BRACED SO THAT THE DISTANCE
BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN LOADS:

SNOW LOAD 25.6 PSF
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

m11,026



Job Track: 43954
Layout ID: 286184
Plan Log: 93671

Builder / Location:

GREENPARK HOMES/ WATERDOWN

Project:

RUSSEL GARDENS PH.2

Date: 8/11/2017

Designer: sonny

Model / Elevation:

BLOCK213 / UNIT1TO4

Mitek ver 7.4.2
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TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.



Delivery Shiplist

DATE	09/12/17
SALES REP	Mario

JOB TRACK:43954 LAYOUT ID: 286185 D LOCATION:
 BUILDER: GREENPARK - RUSSELL GARDENS PH 2 SUB-BUILDER:
 MODEL: DEWBERRY 2E OR DEWBERRY 2ES ELEVATION: 1- UNIT1 OR 1(REV)-UNIT4

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	T1Z HALF HIP	8.00 0.00	21-00-08	04-01-04	2 X 6	2 X 6	01-03-08 00-00-00	01-04-13 04-01-04	121.25 75.34		
	1	T1Z3 HALF HIP	8.00 0.00	21-00-08	04-01-04	2 X 6	2 X 6	01-03-08 00-00-00	01-04-13 04-01-04	121.25 75.34		
	2	T2 HALF HIP	8.00 0.00	21-00-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-01-04	178.24 112.66		
	2	T3 HALF HIP	8.00 0.00	21-00-08	06-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 06-01-04	179.12 112.00		
	2	T4 HALF HIP	8.00 0.00	21-00-08	07-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 07-01-04	188.76 118.68		
	2	T5 HALF HIP	8.00 0.00	21-00-08	08-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 08-01-04	203.36 127.66		
	2	T6 HALF HIP	8.00 0.00	21-00-08	09-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 09-01-04	214.60 134.34		
	4	T7 HALF HIP	8.00 0.00	21-00-08	10-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 10-01-04	448.48 280.00		
	1	G8 COMMON	8.00 0.00	13-02-00	05-09-08	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	55.88 36.00		
	1	T27A MONOPITCH	8.00 0.00	12-09-08	09-11-02	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 09-11-02	61.29 38.50		
	2	T28A MONOPITCH	8.00 0.00	12-06-08	11-03-02	2 X 4	2 X 4	00-00-00 00-00-00	02-10-13 11-03-02	128.48 80.34		
	3	T26 MONOPITCH	8.00 0.00	10-02-08	08-02-08	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 08-02-08	149.52 95.49		
	1	G26 MONOPITCH	8.00 0.00	10-02-08	08-02-08	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 08-02-08	48.89 31.50		
	9	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 00-04-11	151.11 96.03		
	5	J90 JACK	5.00 0.00	05-05-00	02-08-07	2 X 4	2 X 4	01-03-08 00-00-00	00-05-06 02-08-07	77.00 50.00		
	6	J92 JACK	4.00 0.00	07-08-00	02-10-10	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-10-10	136.62 87.00		

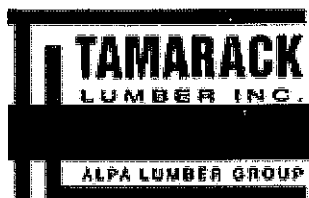
TOTAL # TRUSS= 44.00

TOTAL BFT OF ALL TRUSSES=

1550.88 BFT. TOTAL WEIGHT OF ALL TRUSSES= 2463.85 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
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Delivery Shiplist

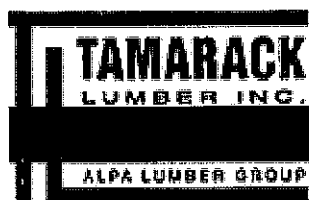
DATE	09/12/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 286185 D LOCATION:
 BUILDER: GREENPARK - RUSSELL GARDENS PH1 SUB-BUILDER:
 MODEL: DEWBERRY 2E OR DEWBERRY 2ES ELEVATION: 1- UNIT1 OR 1(REV)-UNIT4

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
5	Hangers	LJS26DS	

TOTAL # ITEMS= 7.00



Delivery Shiplist

DATE	09/12/17
SALES REP	Mario

JOB TRACK: 43954	LAYOUT ID: 286186	LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH 2	SUB-BUILDER:	
MODEL: DEWBERRY 1	ELEVATION: 1(REV)-UNIT2	

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	6	T29A COMMON	6.00 0.00	31-05-00	10-04-12	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 03-11-00	795.18 497.04		
	1	T30 ROOF	6.00 0.00	31-05-00	09-11-02	2 X 4	2 X 4	00-00-00 00-00-00	03-07-13 05-03-08	152.22 96.17		
	1	T31 ROOF	6.00 0.00	31-05-00	11-01-00	2 X 4	2 X 4	00-00-00 00-00-00	04-11-13 05-03-08	149.81 93.83		
	1	T32 ROOF	6.00 0.00	31-05-00	10-03-08	2 X 4	2 X 4	00-00-00 00-00-00	06-03-13 05-03-08	167.81 105.67		
	1	T33 ROOF	6.00 0.00	31-05-00	10-11-08	2 X 4	2 X 4	00-00-00 00-00-00	07-07-13 05-03-08	161.38 101.67		
	1	T35 HALF HIP	8.00 0.00	19-03-00	04-01-04	2 X 6	2 X 6	00-00-00 00-00-00	01-07-13 04-01-04	110.63 67.34		
	1 3 Ply	T36 HALF HIP	8.00 0.00	19-03-00	05-01-04	2 X 6	2 X 6	00-00-00 00-00-00	01-07-13 05-01-04	367.41 227.01		
	1	G27A GABLE	8.00 0.00	12-09-08	09-11-02	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 09-11-02	68.69 44.00		
	1	T27A MONOPITCH	8.00 0.00	12-09-08	09-11-02	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 09-11-02	61.29 38.50		
	2	T28A MONOPITCH	8.00 0.00	12-06-08	11-03-02	2 X 4	2 X 4	00-00-00 00-00-00	02-10-13 11-03-02	128.48 80.34		
	1	T21 COMMON	8.00 0.00	10-07-00	04-11-02	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	44.34 28.17		
	1	G21 GABLE	8.00 0.00	10-07-00	04-11-02	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	46.02 31.17		
	1 3 Ply	T360A MONOPITCH	8.00 0.00	10-02-08	08-02-08	2 X 6	2 X 6	00-00-00 00-00-00	01-04-13 08-02-08	214.14 135.00		
	3	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 00-04-11	50.37 32.01		
	5	J90 JACK	5.00 0.00	05-05-00	02-08-07	2 X 4	2 X 4	01-03-08 00-00-00	00-05-06 02-08-07	77.00 50.00		
	6	J91 JACK	4.00 0.00	05-08-00	02-02-10	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-02-10	89.64 55.98		

TOTAL # TRUSS= 37.00

TOTAL BFT OF ALL TRUSSES=

1683.90 BFT. TOTAL WEIGHT OF ALL TRUSSES= 2684.41 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
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Delivery Shiplist

DATE	09/12/17
SALES REP	Mario

JOB TRACK:43954	LAYOUT ID: 286186	LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH 2	SUB-BUILDER:	
MODEL: DEWBERRY 1	ELEVATION: 1(REV)-UNIT2	

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	HGUS26-2	
17	Hangers	LJS26DS	

TOTAL # ITEMS= 18.00



Delivery Shiplist

DATE	09/12/17
SALES REP	Mario

JOB TRACK:43954 LAYOUT ID: 286187 LOCATION:
 BUILDER: GREENPARK - RUSSELL GARDENS PH2 SUB-BUILDER:
 MODEL: DEWBERRY1 ELEVATION: 1-UNIT3

ROOF TRUSSES

ROOF TRUSS SPACING:24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	6	T37A COMMON	6.00 0.00	29-01-00	10-04-12	2 X 4	2 X 4	00-00-00 00-00-00	00-11-08 05-03-08	759.18 474.00		
	1	T38 ROOF	6.00 0.00	29-01-00	09-11-02	2 X 4	2 X 4	00-00-00 00-00-00	03-07-13 05-03-08	145.52 92.50		
	1	T39 ROOF	6.00 0.00	29-01-00	11-01-00	2 X 4	2 X 4	00-00-00 00-00-00	04-11-13 05-03-08	140.56 88.67		
	1	T40 ROOF	6.00 0.00	29-01-00	10-11-08	2 X 4	2 X 4	00-00-00 00-00-00	06-03-13 05-03-08	143.96 89.67		
	1	T41 ROOF	6.00 0.00	29-01-00	10-11-08	2 X 4	2 X 4	00-00-00 00-00-00	07-07-13 05-03-08	155.00 98.00		
	1	T42 FLAT GIRDER	0.00 0.00	19-03-00	04-01-04	2 X 6	2 X 6	00-00-00 00-00-00	04-01-04 04-01-04	115.97 72.00		
	1 3 Ply	T43 FLAT GIRDER	0.00 0.00	19-03-00	05-01-04	2 X 6	2 X 6	00-00-00 00-00-00	05-01-04 05-01-04	377.70 236.01		
	1 3 Ply	T44 ROOF	8.00 0.00	19-07-08	08-02-08	2 X 4	2 X 6	00-00-00 00-00-00	01-04-13 08-02-08	325.05 208.98		
	2	T26 MONOPITCH	8.00 0.00	10-02-08	08-02-08	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 08-02-08	99.68 63.66		
	1	G21 GABLE	8.00 0.00	10-07-00	04-11-02	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	46.02 31.17		
	1	G27A GABLE	8.00 0.00	12-09-08	09-11-02	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 09-11-02	68.69 44.00		
	1	T27A MONOPITCH	8.00 0.00	12-09-08	09-11-02	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 09-11-02	61.29 38.50		
	2	T28A MONOPITCH	8.00 0.00	12-06-08	11-03-02	2 X 4	2 X 4	00-00-00 00-00-00	02-10-13 11-03-02	128.48 80.34		
	3	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 00-04-11	50.37 32.01		
	5	J4 JACK-OPEN	6.00 0.00	01-11-08	02-01-12	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 02-01-12	36.65 23.35		
	5	J90 JACK	5.00 0.00	05-05-00	02-08-07	2 X 4	2 X 4	01-03-08 00-00-00	00-05-06 02-08-07	77.00 50.00		
	6	J91 JACK	4.00 0.00	05-08-00	02-02-10	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-02-10	89.64 55.98		

TOTAL # TRUSS= 43.00

TOTAL BFT OF ALL TRUSSES=

1778.84 BFT. TOTAL WEIGHT OF ALL TRUSSES= 2820.76 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
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Delivery Shiplist

DATE	09/12/17
SALES REP	Mario

JOB TRACK: 43954	LAYOUT ID: 286187	LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH2	SUB-BUILDER:	
MODEL: DEWBERRY1	ELEVATION: 1-UNIT3	

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	HGUS26-2	
22	Hangers	LJS26DS	

TOTAL # ITEMS= 23.00

ID:LjPBf7XlyxdQBIYCH4AXRGyehXO-iUZX6EHNILMgfH0NWnQKMHU4mUmbIgLWgJEHEAzsY0Y



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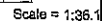
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
286185	T1Z	1	1	TRUSS DESC.	

Version 8.200 S Dec 12 2017 MITek Industries, Inc. Tue Jan 23 08:32:11 2018 Page 2
 Tamarack Roof Truss, Burlington ID:LJPBf7XlyxdQBIYCH4AXRGrehXO-IUZx6EHNILMqfth0NWnQKMfHU4mUmbIqLWqJEHEAzsY0Y

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION
 (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 616 354 1667 822 2284 1656
 PLATE PLACEMENT TOL. = 0.250 inches
 PLATE ROTATION TOL. = 5.0 Deg.
 JSI GRIP= 0.90 (L) (INPUT = 0.90)
 JSI METAL= 0.87 (J) (INPUT = 1.00)



DWG NO. TAM 5099-18
 STRUCTURAL
 COMPONENT ONLY



DRY: SEASONED LUMBER.

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

EDDING BEARINGS

UNFACTORED REACTIONS

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.08 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)



SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN, C/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/999 (0.09")

CSI: TC=0.52/1.00 (B-C:1), BC=0.33/1.00 (J-K:2),
WB=0.31/1.00 (F-H:1), SSI=0.21/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

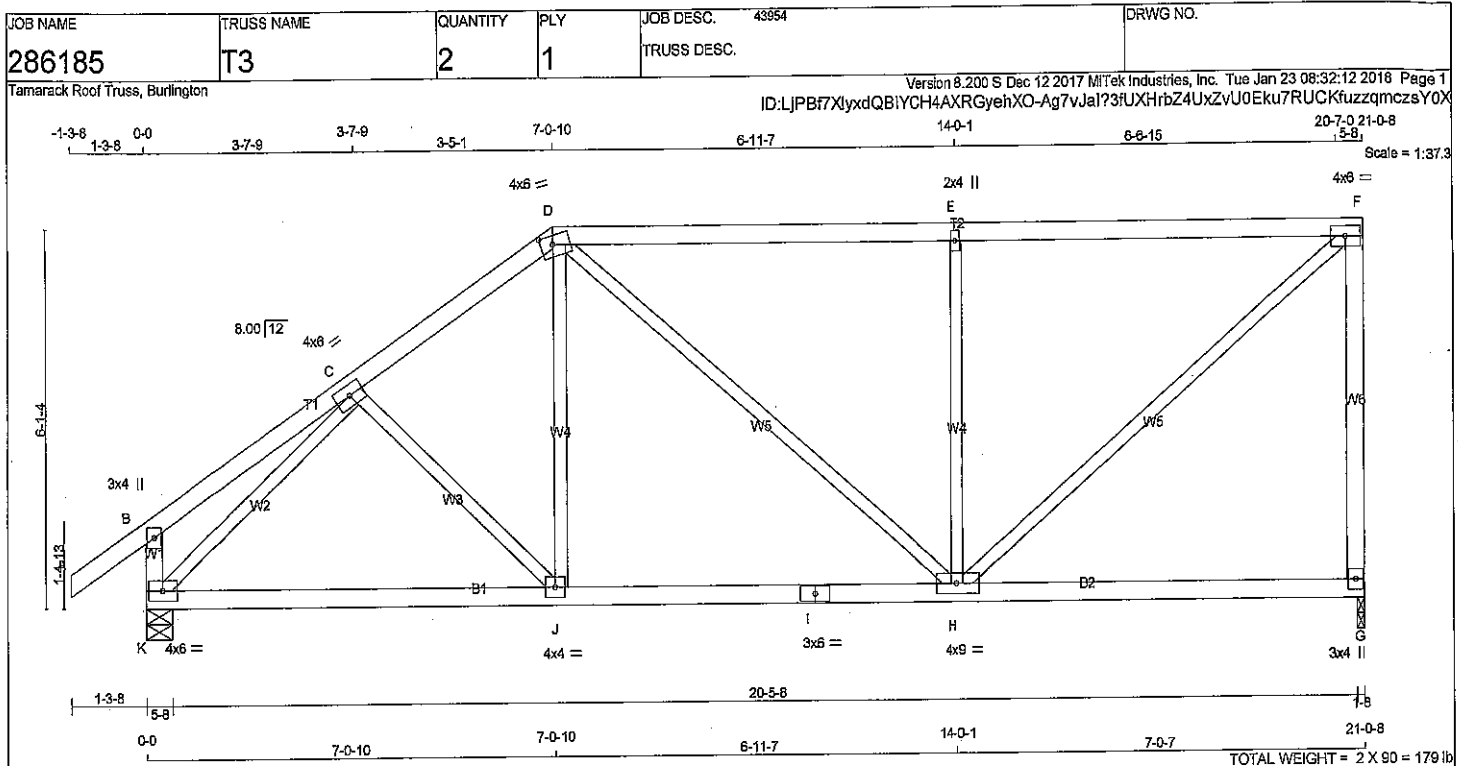
PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg

JSI GRIP= 0.86 (K) (INPUT = 0.90)
JSI METAL= 0.38 (K) (INPUT = 1.00)

DWG NO. TAM 5101 -18
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF EXCEPT

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-l	MT20	4.0	6.0		
D	TTWW-m	MT20	4.0	8.0	1.75	2.50
E	TMW+w	MT20	2.0	4.0		
F	TMW-l	MT20	4.0	8.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-l	MT20	4.0	8.0		
I	BS-l	MT20	3.0	8.0		
J	BMW-l	MT20	4.0	4.0		
K	BMVW-l	MT20	4.0	8.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	UP/LIFT		
G	1181	0	1181	0	1-8	1-8
K	1297	0	1297	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE		MAX / MIN COMPONENT REACTIONS		DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE		
G	970	538 / 0	221 / 0	0 / 0	210 / 0	0 / 0
K	1048	609 / 0	221 / 0	0 / 0	219 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.74 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		MAX. UNBRAC LENGTH	MEMB.	WEBS		MAX. FACTORED
	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX. CSI (LC)			MAX. FORCE (LBS)	MAX. CSI (LC)	
FR-TO						FR-TO			
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	C-J	-47 / 93	0.02 (3)	
B-C	0 / 18	-84.3	-84.3	0.18 (1)	10.00	J-D	0 / 336	0.08 (2)	
C-D	-1192 / 0	-84.3	-84.3	0.18 (1)	5.85	K-C	-1408 / 0	0.54 (1)	
D-E	-1059 / 0	-84.3	-84.3	0.78 (1)	4.74	H-F	0 / 1374	0.31 (1)	
E-F	-1059 / 0	-84.3	-84.3	0.78 (1)	4.74	D-H	0 / 103	0.02 (1)	
G-F	-1100 / 0	0.0	0.0	0.79 (1)	7.54	H-E	-731 / 0	0.43 (1)	
K-B	-232 / 0	0.0	0.0	0.02 (1)	7.81				
K-J	0 / 1010	-28.0	-28.0	0.44 (2)	10.00				
J-I	0 / 980	-28.0	-28.0	0.44 (2)	10.00				
I-H	0 / 980	-28.0	-28.0	0.44 (2)	10.00				
H-G	0 / 0	-28.0	-28.0	0.31 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	PSF
LL	25.6	PSF
BOT CH.	LL	PSF
LL	10.5	PSF
DL	7.0	PSF
TOTAL LOAD	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL) = L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/999 (0.18")

CSI: TC=0.78/1.00 (F-G:1), BC=0.44/1.00 (H-J:2), WB=0.54/1.00 (C-K:1), SS=0.29/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

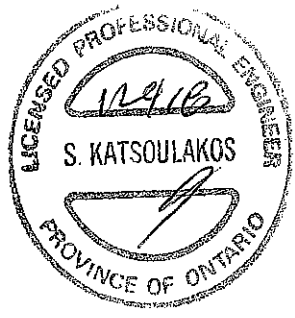
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	822	2264	1656

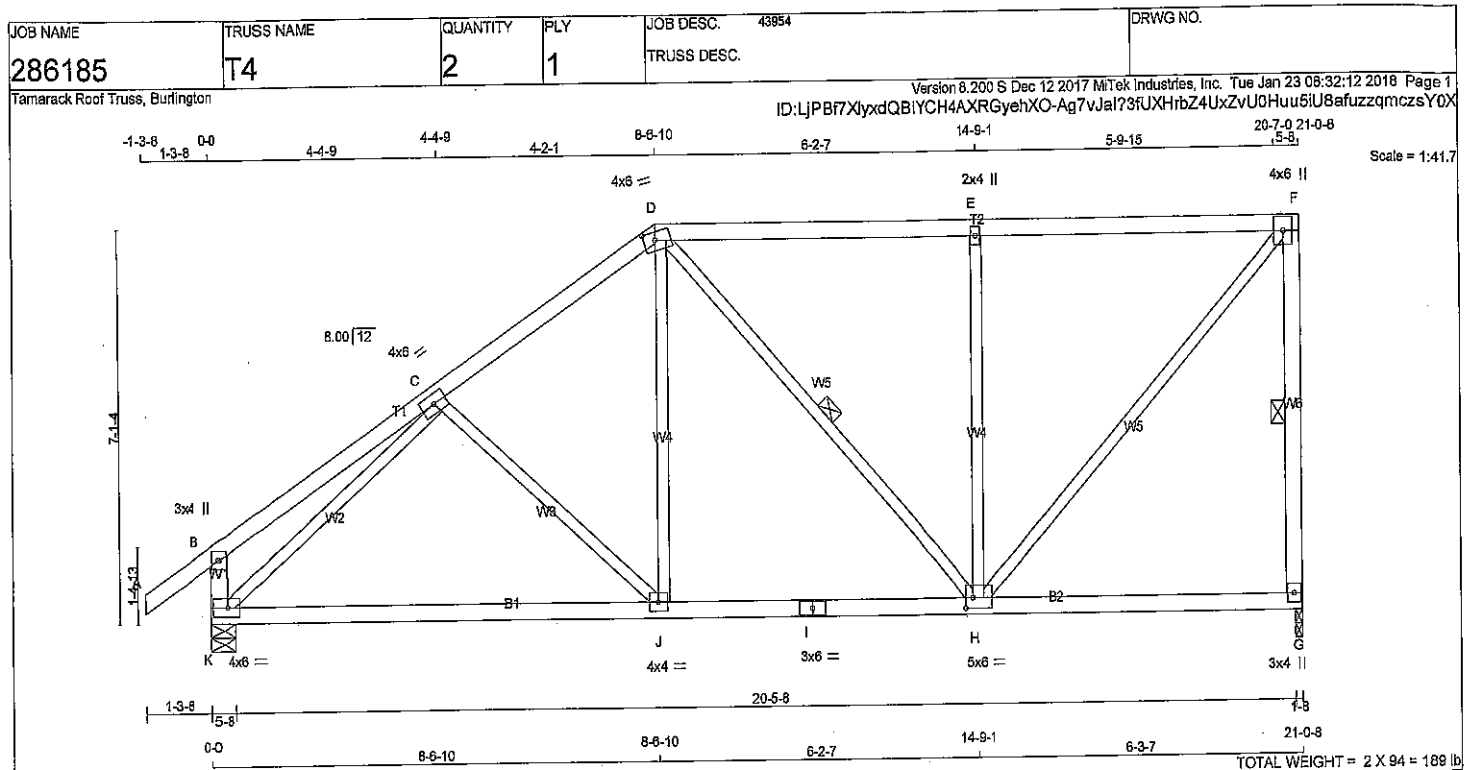
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (F) (INPUT = 0.90)
JSI METAL= 0.34 (C) (INPUT = 1.00)



DWG NO. TAM 5102-18
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES - CHORDS

MEMB.	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	4.0	8.0		
D	TTWW-m	MT20	4.0	6.0	1.75	2.50
E	TMVW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMVW-t	MT20	3.0	4.0		
H	BMVWW-t	MT20	5.0	6.0	2.25	1.50
I	BS-t	MT20	3.0	8.0		
J	BMVW-t	MT20	4.0	4.0		
K	BMVW-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
G	1181	0	1181	0	1-8	1-8
K	1297	0	1297	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	970	538 / 0	221 / 0	0 / 0	0 / 0	210 / 0	0 / 0
K	1048	609 / 0	221 / 0	0 / 0	0 / 0	219 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.67 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CS (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS (LC)
FR-TO					FR-TO		
A-B	0 / 32	-84.3	-84.3 0.11 (1)	10.00	C-J	-180 / 63	0.10 (1)
B-C	0 / 23	-84.3	-84.3 0.24 (1)	10.00	J-D	0 / 428	0.10 (2)
C-D	-1112 / 0	-84.3	-84.3 0.27 (1)	5.87	K-C	-1392 / 0	0.78 (1)
D-E	-834 / 0	-84.3	-84.3 0.59 (1)	5.68	H-F	0 / 1231	0.26 (1)
E-F	-835 / 0	-84.3	-84.3 0.59 (1)	5.68	D-H	-137 / 0	0.08 (2)
G-F	-1108 / 0	0.0	0.0 0.25 (1)	6.02	H-E	-852 / 0	0.57 (1)
K-B	-254 / 0	0.0	0.0 0.03 (1)	7.81			
K-J	0 / 1025	-28.0	-28.0 0.55 (2)	10.00			
J-I	0 / 910	-28.0	-28.0 0.55 (2)	10.00			
I-H	0 / 910	-28.0	-28.0 0.55 (2)	10.00			
H-G	0 / 0	-28.0	-28.0 0.24 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.I. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/999 (0.22")
ALLOWABLE DEFL.(TL) = L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/678 (0.37")

CSI: TC=0.59/1.00 (E-F:1), BC=0.55/1.00 (H-J:2), WB=0.78/1.00 (C-K:1), SS=0.26/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1856

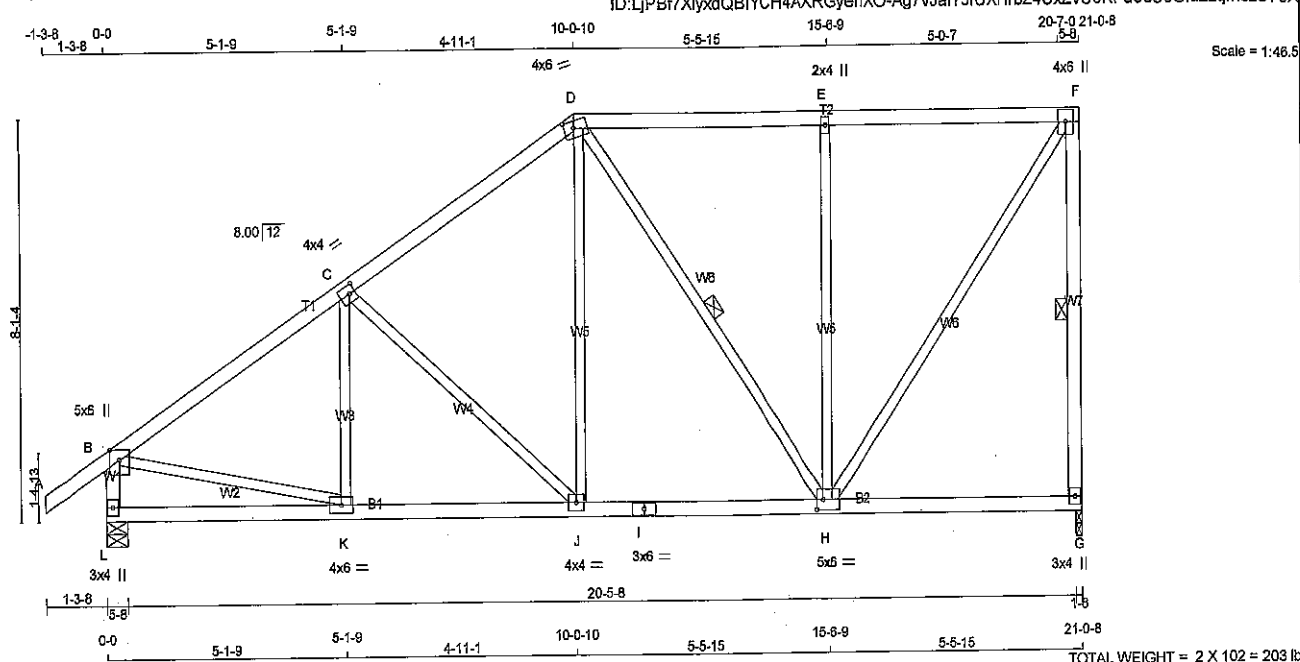
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (H) (INPUT = 0.90)
JSI METAL= 0.33 (C) (INPUT = 1.00)



DWG NO. TAM 5103 -18
STRUCTURAL
COMPONENT ONLY



LUMBER		LUMBER		DESCR
N. L. G. A. RULES				
CHORDS	SIZE			
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
L - G	2x4	DRY	No.2	SPF

ALL WEBS	2x3	DRY
EXCEPT		

DRY: SEASONED LUMBER.

PLATES (table is in inches)		W	LEN	Y	X
JT	TYPE	PLATES			
B	TMWV+p	MT20	5.0	6.0	Edge
C	TMWW-t	MT20	4.0	4.0	2.00 1.50
D	TTWV-m	MT20	4.0	8.0	1.75 2.50
E	TMW+w	MT20	2.0	4.0	
F	TMWV+p	MT20	4.0	6.0	
G	BMV1+p	MT20	3.0	4.0	
H	BMWWWW-t	MT20	5.0	8.0	2.50 1.50
I	BS-t	MT20	3.0	6.0	
J	BMWW-t	MT20	4.0	4.0	
K	BMWW-t	MT20	4.0	6.0	
L	BMV1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BUILDING BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1181	0	1181	0	0	1-8	1-8
L	1297	0	1297	0	0	5-8	5-8

UNFACTORED REACTIONS

UNFROSTED REACTIONS		MAX./MIN. COMPONENT REACTIONS					
JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	970	538 / 0	221 / 0	0 / 0	0 / 0	210 / 0	0 / 0
L	1048	609 / 0	221 / 0	0 / 0	0 / 0	219 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.19 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED			
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRAC	MEMB.	FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
A-B	0/32	-84.3	-84.3 0.11 (1)	10.00	K-C	-72/167	0.04 (3)
B-C	-1281/0	-84.3	-84.3 0.41 (1)	5.19	C-J	-391/0	0.33 (1)
C-D	-988/0	-84.3	-84.3 0.39 (1)	5.74	J-D	0/418	0.09 (2)
D-E	-866/0	-84.3	-84.3 0.44 (1)	6.25	D-H	-238/0	0.15 (1)
E-F	-868/0	-84.3	-84.3 0.44 (1)	6.25	H-E	-573/0	0.73 (1)
G-F	-1118/0	0.0	0.0 0.33 (1)	5.99	H-F	0/1153	0.26 (1)
L-B	-1237/0	0.0	0.0 0.13 (1)	7.22	B-K	0/1113	0.25 (1)
L-K	0/0	-28.0	-28.0 0.17 (3)	10.00			
K-J	0/1089	-28.0	-28.0 0.30 (2)	10.00			
J-I	0/803	-28.0	-28.0 0.30 (2)	10.00			
I-H	0/803	-28.0	-28.0 0.30 (2)	10.00			
H-G	0/0	-28.0	-28.0 0.21 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL	=	25.8	PSF
		DL	=	3.0	PSF
BOT	CH.	LL	=	10.5	PSF
		DL	=	7.0	PSF
TOTAL LOAD				=	46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/999 (0.07")

CSI: TC=0.44/1.00 (E-F:1), BC=0.30/1.00 (H-J:2),
WB=0.73/1.00 (E-H:1), SSI=0.23/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

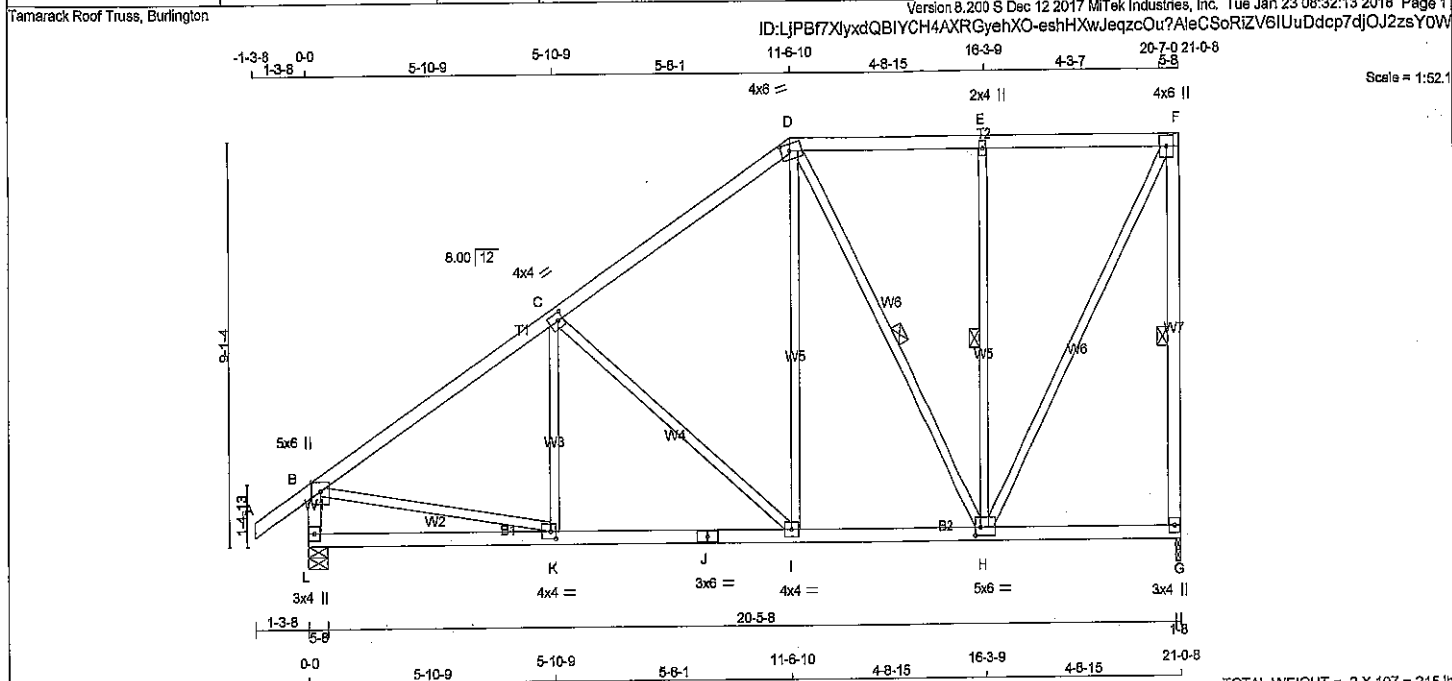
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.27 (K) (INPUT = 1.00)



DWG NO. TAM 5104 -18
STRUCTURAL
COMPONENT ONLY

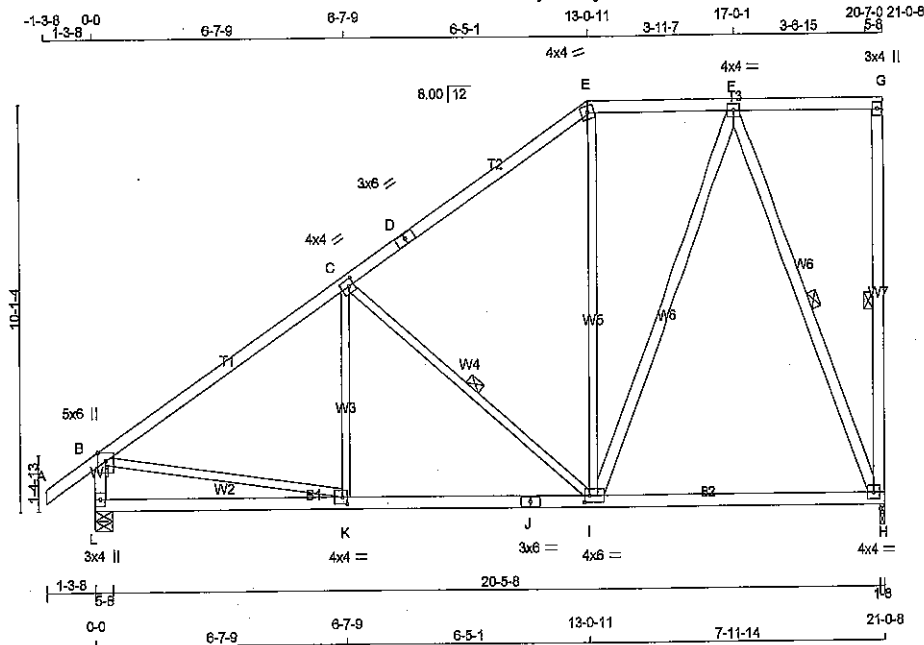
JOB NAME 286185	TRUSS NAME T6	QUANTITY 2	PLY 1	JOB DESC. 43954	DRWG NO.
Tamarack Roof Truss, Burlington					



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
286185	T7	4	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1:57.5

TOTAL WEIGHT = 4 X 112 = 448 lb

LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER		
A - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
H - G	2x4	DRY	No.2	SPF	
L - B	2x4	DRY	No.2	SPF	
L - J	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
I - F	2x4	DRY	No.2	SPF	
F - H	2x4	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	HORZ	GROSS REACTION	DOWN	HORZ	BRG	IN-SX	BRG	IN-SX
H	1181	0	1181	0	0	1-8	1-8		
L	1297	0	1297	0	0	5-8	5-8		

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN. COMPONENT REACTIONS		LIVE		PERM. LIVE		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	GROSS	VERT	HORZ	BRG	IN-SX	BRG	IN-SX	BRG	IN-SX	BRG	IN-SX
H	970	538 / 0	221 / 0	0 / 0	0 / 0	0 / 0	210 / 0	0 / 0	210 / 0	0 / 0	0 / 0	0 / 0	0 / 0
L	1048	609 / 0	221 / 0	0 / 0	0 / 0	0 / 0	219 / 0	0 / 0	219 / 0	0 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.07 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-H, C-I, F-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 32	K-C	0 / 250
B-C	-1253 / 0	C-I	-808 / 0
C-D	-770 / 0	I-E	0 / 187
D-E	-770 / 0	E-H	0 / 588
E-F	-809 / 0	F-H	-1040 / 0
F-G	0 / 0	B-K	0 / 1087
H-G	-129 / 0		
L-B	-1219 / 0		
L-K	0 / 0		
K-J	0 / 1073		
J-I	0 / 1073		
I-H	0 / 395		

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 32	K-C	0 / 250
B-C	-1253 / 0	C-I	-808 / 0
C-D	-770 / 0	I-E	0 / 187
D-E	-770 / 0	E-H	0 / 588
E-F	-809 / 0	F-H	-1040 / 0
F-G	0 / 0	B-K	0 / 1087
H-G	-129 / 0		
L-B	-1219 / 0		
L-K	0 / 0		
K-J	0 / 1073		
J-I	0 / 1073		
I-H	0 / 395		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.8 PSF
DL	= 3.0 PSF
BOT CH. LL	= 10.5 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL)= L/999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL)= L/999 (0.25")

CSI: TC=0.52/1.00 (B-C:1), BC=0.51/1.00 (I-K:2), WB=0.62/1.00 (F-H:1), SS=0.22/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES		PLATE GRIP(DRY) SHEAR		SECTION	
(PSI)	(PLI)	(PSI)	(PLI)	(PSI)	(PLI)
MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1887	822	2284

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (K) (INPUT = 0.90)
JSI METAL= 0.40 (K) (INPUT = 1.00)

DRWG NO. TAM 5006-18
STRUCTURAL
COMPONENT ONLY

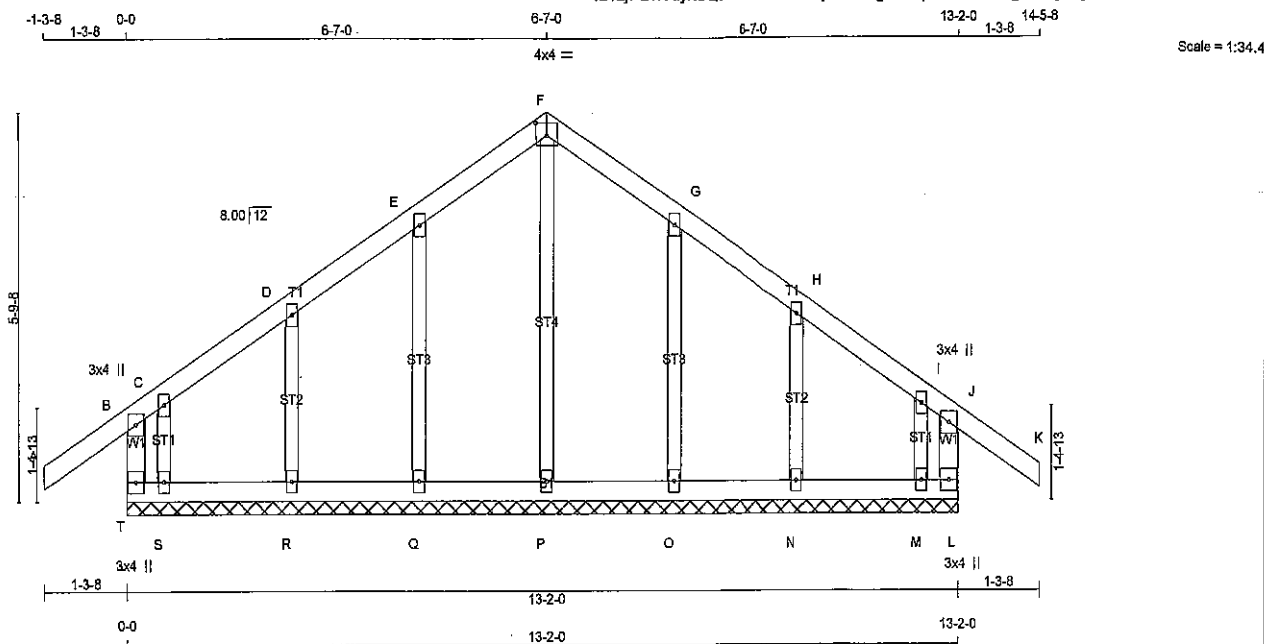


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
286184	G8	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 56 = 112 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
T - B	2x4	DRY	No.2	SPF	
A - F	2x4	DRY	No.2	SPF	
F - K	2x4	DRY	No.2	SPF	
L - J	2x4	DRY	No.2	SPF	
T - L	2x4	DRY	No.2	SPF	

ALL WEBS	2x3	DRY	No.2	SPF	
ALL GABLE WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

GABLE STUDS SPACED AT 2'-0" O.C.

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE PLATES	3.0	4.0		
B TMV+p MT20	3.0	4.0		
C, D, E, G, H, I	2.0	4.0	2.25	2.00
F TMV+w MT20	4.0	4.0		
J TMV+p MT20	3.0	4.0		
L BMV1+p MT20	3.0	4.0		
M, N, O, P, Q, R, S	2.0	4.0		
M BMV1+w MT20	2.0	4.0		
T BMV1+p MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
T-B	-209 / 0	0.0	0.0 0.03 (1)	7.81	P-F	-201 / 0	0.10 (1)
A-B	0 / 32	-84.3	-84.3 0.11 (1)	10.00	Q-E	-171 / 0	0.05 (1)
B-C	-31 / 0	-84.3	-84.3 0.06 (1)	6.25	R-D	-169 / 0	0.03 (1)
C-D	0 / 17	-84.3	-84.3 0.04 (1)	10.00	S-C	-20 / 0	0.00 (1)
D-E	0 / 21	-84.3	-84.3 0.04 (1)	10.00	O-G	-171 / 0	0.05 (1)
E-F	0 / 24	-84.3	-84.3 0.04 (1)	10.00	N-H	-169 / 0	0.03 (1)
F-G	0 / 24	-84.3	-84.3 0.04 (1)	10.00	M-I	-20 / 0	0.00 (1)
G-H	0 / 21	-84.3	-84.3 0.04 (1)	10.00			
H-I	0 / 17	-84.3	-84.3 0.04 (1)	10.00			
I-J	-31 / 0	-84.3	-84.3 0.08 (1)	6.25			
J-K	0 / 32	-84.3	-84.3 0.11 (1)	10.00			
L-J	-209 / 0	0.0	0.0 0.03 (1)	7.81			
T-S	-11 / 0	-28.0	-28.0 0.02 (3)	6.25			
S-R	-12 / 0	-28.0	-28.0 0.02 (2)	6.25			
R-Q	-17 / 0	-28.0	-28.0 0.02 (2)	6.25			
Q-P	-21 / 0	-28.0	-28.0 0.02 (2)	6.25			
P-O	-21 / 0	-28.0	-28.0 0.02 (2)	6.25			
O-N	-17 / 0	-28.0	-28.0 0.02 (2)	6.25			
N-M	-12 / 0	-28.0	-28.0 0.02 (2)	6.25			
M-L	-11 / 0	-28.0	-28.0 0.02 (3)	6.25			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
DL =	3.0	PSF	
BOT CH.	LL =	10.5	PSF
DL =	7.0	PSF	
TOTAL LOAD =	46.1	PSF	

SPACING = 24.0 IN. GIC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11 (A-B:1), BC=0.02 (Q-R:2), WB=0.10 (F-P:1), SSI=0.08 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1687 822 2284 1656

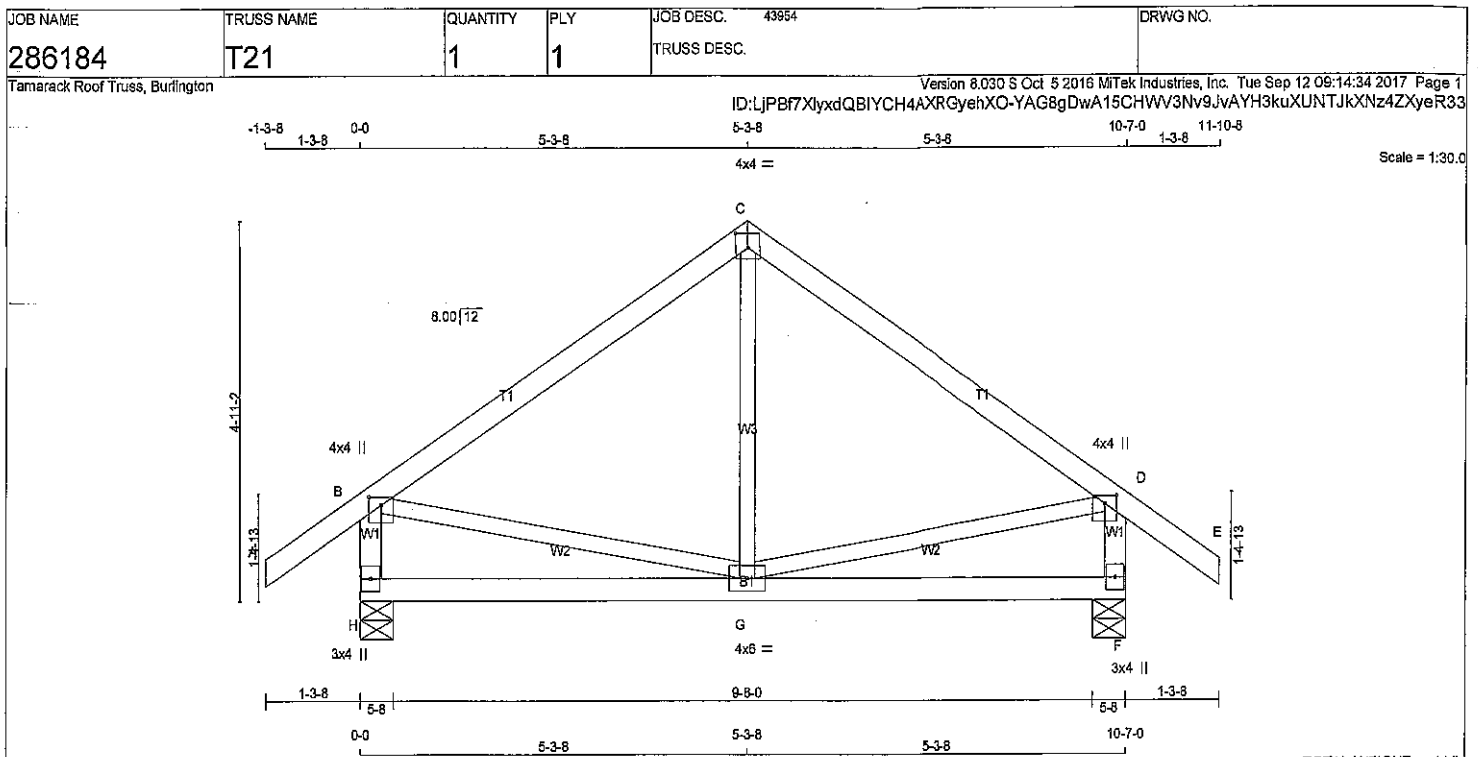
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.17 (F) (INPUT = 0.90)
JSI METAL= 0.05 (F) (INPUT = 1.00)



DWNO.YAM 46203 17
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY; SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMVW+p	MT20	4.0	4.0	1.25	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMVWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ UPLIFT		
H	710	0	710	0	5-8	5-8
F	710	0	710	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	566	341 / 0	111 / 0	0 / 0	0 / 0	114 / 0	0 / 0
F	566	341 / 0	111 / 0	0 / 0	0 / 0	114 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	CHORDS		MAX. MEMB. LENGTH FR-TO	WEBS	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
		VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)				
FR-TO							
A-B	0 / 32	-84.3	-84.3 0.11 (1)	10.00	G-C	0 / 236	0.05 (3)
B-C	-434 / 0	-84.3	-84.3 0.30 (1)	6.25	B-G	0 / 366	0.08 (1)
C-D	-434 / 0	-84.3	-84.3 0.30 (1)	6.25	G-D	0 / 366	0.08 (1)
D-E	0 / 32	-84.3	-84.3 0.11 (1)	10.00			
H-B	-653 / 0	0.0	0.0 0.07 (1)	7.81			
F-D	-653 / 0	0.0	0.0 0.07 (1)	7.81			
H-G	0 / 0	-28.0	-28.0 0.23 (3)	10.00			
G-F	0 / 0	-28.0	-28.0 0.23 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(5% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/989 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/898 (0.04")

CSI: TC=0.30 (C-D:1), BC=0.23 (G-H:3), WB=0.08 (B-G:1), SSI=0.14 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

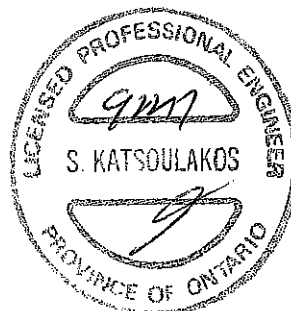
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1658

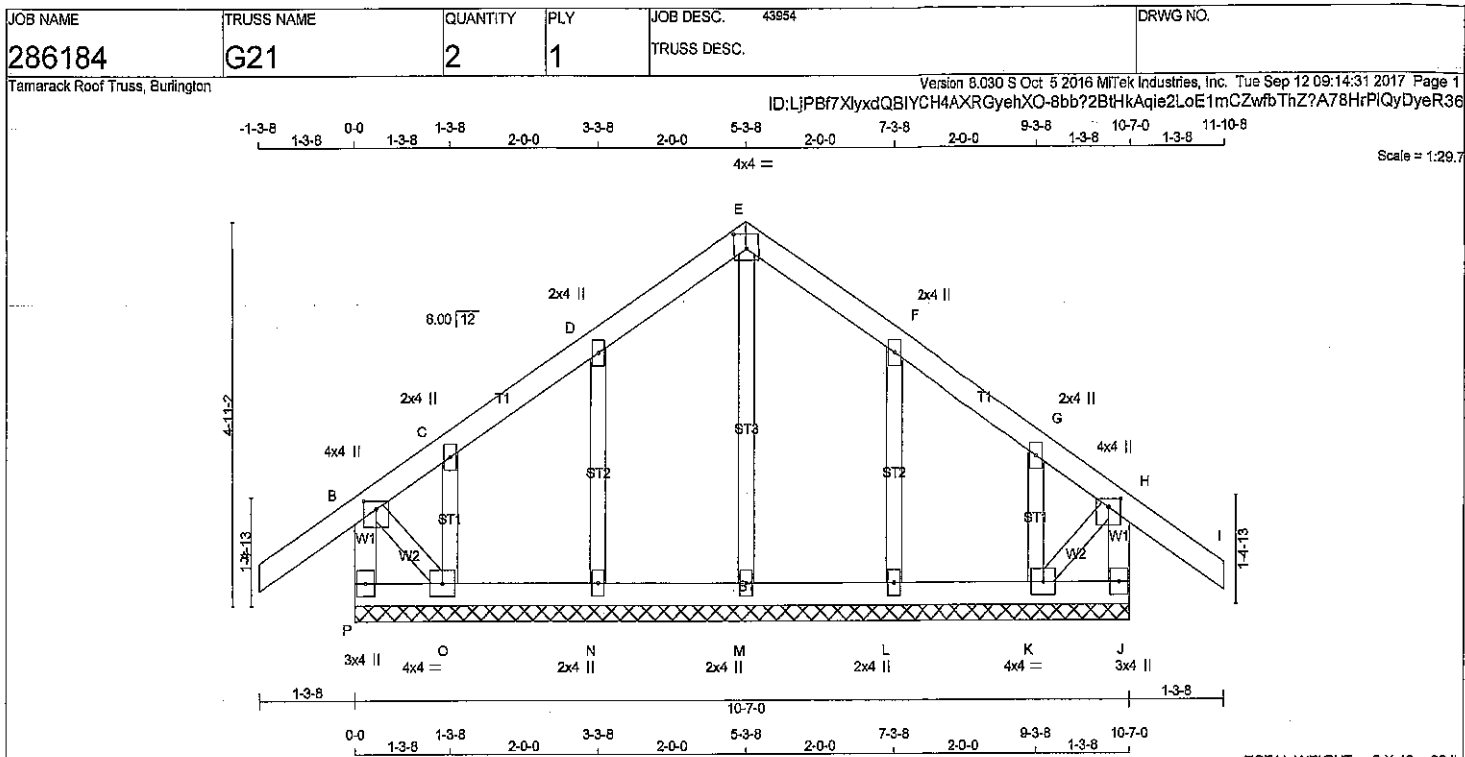
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.51 (G) (INPUT = 0.90)
JSI METAL= 0.15 (B) (INPUT = 1.00)



DRW NO. TAM 46102-17
STRUCTURAL
COMPONENT ONLY



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - E	2x4	DRY	No.2
E - I	2x4	DRY	No.2
P - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
P - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			
GABLE STUDS SPACED AT	2-0-0	OC.	

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	4.0	1.25	2.00
C, D, F, G					
C TMVW+w	MT20	2.0	4.0		
E TTW+p	MT20	4.0	4.0	2.25	2.00
H TMVW+p	MT20	4.0	4.0	1.25	2.00
J BMV1+p	MT20	3.0	4.0		
K BMWW1-t	MT20	4.0	4.0		
L, M, N					
L BMW1+w	MT20	2.0	4.0		
O BMWW1-t	MT20	4.0	4.0		
P BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH LC1 (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0/32	-84.3 -84.3	0.11 (1)	10.00	M-E	-133/0 0.05 (1)	
B-C	-45/0	-84.3 -84.3	0.11 (1)	8.25	N-D	-205/0 0.04 (1)	
C-D	0/0	-84.3 -84.3	0.05 (1)	10.00	O-C	-57/0 0.01 (1)	
D-E	-14/0	-84.3 -84.3	0.05 (1)	8.25	L-F	-205/0 0.04 (1)	
E-F	-14/0	-84.3 -84.3	0.05 (1)	8.25	K-G	-57/0 0.01 (1)	
F-G	0/0	-84.3 -84.3	0.05 (1)	10.00	B-O	0/13 0.00 (1)	
G-H	-45/0	-84.3 -84.3	0.11 (1)	8.25	K-H	0/13 0.00 (1)	
H-I	0/32	-84.3 -84.3	0.11 (1)	10.00			
P-B	-241/0	0.0 0.0	0.02 (1)	7.81			
J-H	-241/0	0.0 0.0	0.02 (1)	7.81			
P-O	0/0	-28.0 -28.0	0.02 (3)	10.00			
O-N	0/8	-28.0 -28.0	0.02 (2)	10.00			
N-M	0/3	-28.0 -28.0	0.02 (3)	10.00			
M-L	0/3	-28.0 -28.0	0.02 (3)	10.00			
L-K	0/8	-28.0 -28.0	0.02 (2)	10.00			
K-J	0/0	-28.0 -28.0	0.02 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11 (A-B:1), BC=0.02 (N-O:2), WB=0.05 (E-M:1), SS=0.08 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

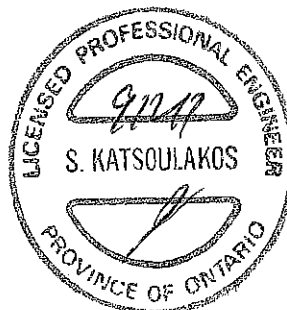
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 922 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.17 (H) (INPUT = 0.90)
JSI METAL= 0.05 (D) (INPUT = 1.00)



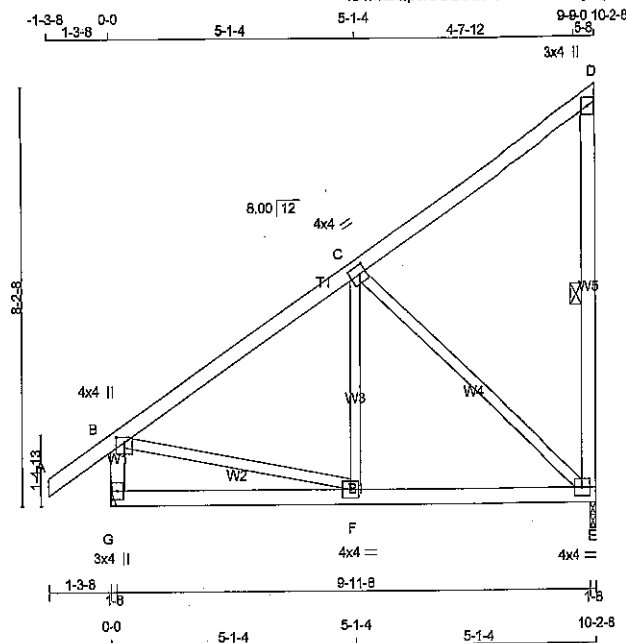
DWG NO. TAM 4620417
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43854	DRWG NO.
286185	T26	3	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1:45.4

TOTAL WEIGHT = 3 X 50 = 150 lb

(M/F)

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TMV+p	MT20	3.0	4.0		
E	BMVW-t	MT20	4.0	4.0		
F	BMVW-t	MT20	4.0	4.0		
G	BMV-t+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

BEARINGS							
	FACTORED		MAXIMUM FACTORED			INPUT	REQ'D
	GROSS REACTION		GROSS REACTION			BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	573	0	573	0	0	1-8	1-8
G	689	0	689	0	0	HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED					
E	470	281 / 0	107 / 0	0 / 0	102 / 0	0 / 0
G	549	331 / 0	107 / 0	0 / 0	110 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-E.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD LC1 (LC)
FR-TO		FROM TO		FR-TO		FROM TO	
A-B	0 / 32	-84.3 -84.3	0.11 (1)	10.00	F-C	0 / 288	0.08 (3)
B-C	-457 / 0	-84.3 -84.3	0.29 (1)	6.25	C-E	-539 / 0	0.44 (1)
C-D	-29 / 0	-84.3 -84.3	0.28 (1)	6.25	B-F	0 / 414	0.09 (1)
E-D	-162 / 0	0.0 0.0	0.05 (1)	6.25			
G-B	-833 / 0	0.0 0.0	0.07 (1)	7.81			
G-F	0 / 0	-28.0 -28.0	0.21 (3)	10.00			
F-E	0 / 405	-28.0 -28.0	0.26 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 25.6	PSF
	DL	= 3.0	PSF
BOT CH.	LL	= 10.5	PSF
	DL	= 7.0	PSF
TOTAL LOAD		= 48.1	PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.29/1.00 (B-C:1), BC=0.26/1.00 (E-F:2),
WB=0.44/1.00 (C-E:1), SS=0.17/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

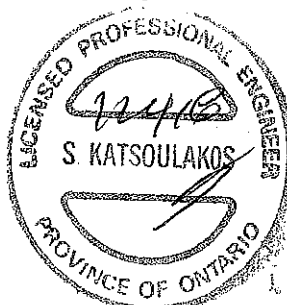
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	818	354	1657
	822	2284	1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.51 (B) (INPUT = 0.90)
JSI METAL= 0.15 (F) (INPUT = 1.00)



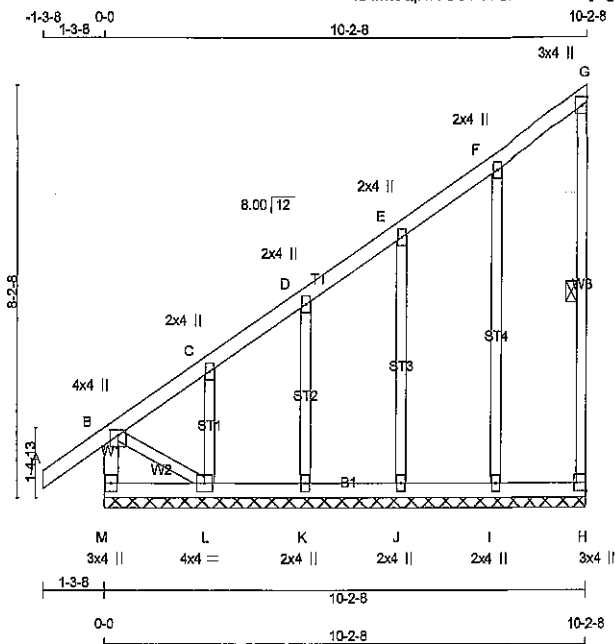
DRWG NO. TAM 5/07-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43854	DRWG NO.
286184	G26	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1:46.0

TOTAL WEIGHT = 2 X 51 = 101 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
M - B	2x4	DRY	No.2	SPF
A - G	2x4	DRY	No.2	SPF
H - G	2x3	DRY	No.2	SPF
M - H	2x4	DRY	No.2	SPF

ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2'-0" OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C, D, E, F						
C	TMVW+w	MT20	2.0	4.0		
G	TMV+p	MT20	3.0	4.0		Edge
H	BMV1+p	MT20	3.0	4.0		2.00 Edge
I, J, K						
I	BMW1+w	MT20	2.0	4.0		
L	BMWW1-t	MT20	4.0	4.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
M-B	-204 / 0	0.0 0.0	0.02 (1)	I-F	-190 / 0	0.15 (1)	
A-B	0 / 32	-84.3 -84.3	0.11 (1)	J-E	-184 / 0	0.08 (1)	
B-C	-8 / 0	-84.3 -84.3	0.05 (1)	K-D	-160 / 0	0.04 (1)	
C-D	-13 / 0	-84.3 -84.3	0.05 (1)	L-C	-194 / 0	0.03 (1)	
D-E	-8 / 0	-84.3 -84.3	0.04 (1)	B-L	0 / 16	0.00 (1)	
E-F	-1 / 0	-84.3 -84.3	0.05 (1)				
F-G	-9 / 0	-84.3 -84.3	0.05 (1)				
H-G	-71 / 0	0.0 0.0	0.03 (1)				
M-L	0 / 0	-28.0 -28.0	0.03 (2)				
L-K	0 / 8	-28.0 -28.0	0.03 (2)				
K-J	0 / 5	-28.0 -28.0	0.02 (2)				
J-I	0 / 2	-28.0 -28.0	0.03 (2)				
I-H	0 / 0	-28.0 -28.0	0.03 (2)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.8	PSF
DL	= 3.0	PSF
BOT CH.	LL = 10.5	PSF
DL	= 7.0	PSF
TOTAL LOAD	= 46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11 (A-B:1), BC=0.03 (K-L:2), WB=0.15 (F-I:1), SSI=0.08 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1887 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (B) (INPUT = 0.90)
JSI METAL= 0.05 (C) (INPUT = 1.00)



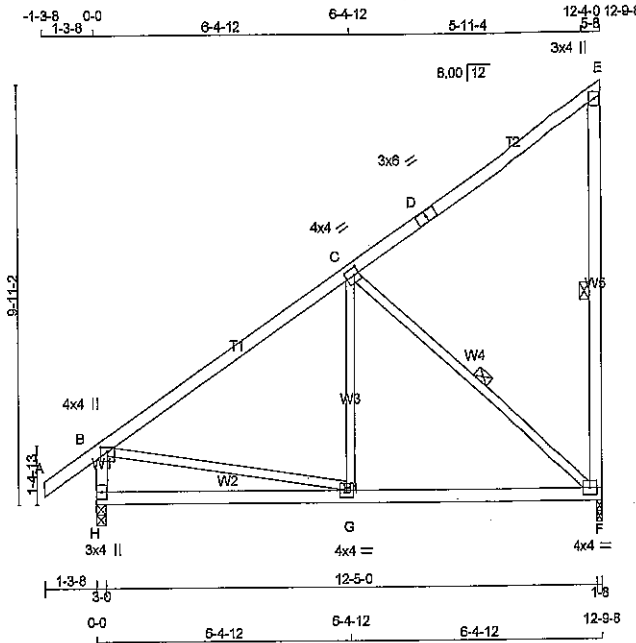
DWG NO. TAM46205 17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
286185	T27A	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1:54.7

TOTAL WEIGHT = 61 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TMV+p	MT20	3.0	4.0		
F	BMVW1-t	MT20	4.0	4.0		
G	BMWW-t	MT20	4.0	4.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	UPLIFT
F	718	0	718	0
H	834	0	834	0

UNFACTORED REACTIONS

JT	1ST CASE	MAX MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	588	327 / 0	134 / 0	0 / 0	0 / 0	0 / 0	126 / 0	0 / 0
H	668	397 / 0	134 / 0	0 / 0	0 / 0	0 / 0	136 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-F, C-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1	MAX LC2	MAX UNBRACED LENGTH	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX LC1	MAX LC2
FR-TO							FR-TO				
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	10.00	G-C	0 / 345	0.08 (3)		
B-C	-805 / 0	-84.3	-84.3	0.46 (1)	6.25	6.25	C-F	-697 / 0	0.32 (1)		
C-D	-37 / 0	-84.3	-84.3	0.45 (1)	6.25	6.25	B-G	0 / 542	0.12 (1)		
D-E	-37 / 0	-84.3	-84.3	0.45 (1)	6.25	6.25					
F-E	-203 / 0	0.0	0.0	0.10 (1)	8.25	8.25					
H-B	-765 / 0	0.0	0.0	0.08 (1)	7.81	7.81					
H-G	0 / 0	-28.0	-28.0	0.33 (3)	10.00	10.00					
G-F	0 / 534	-28.0	-28.0	0.39 (2)	10.00	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.43")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.05")
ALLOWABLE DEFL.(TL) = $L/360$ (0.43")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.09")

CSI: TC=0.48/1.00 (B-C:1), BC=0.39/1.00 (F-G:2), WB=0.32/1.00 (C-F:1), SSI=0.22/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

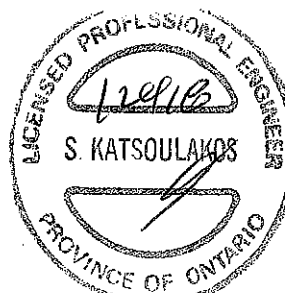
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	618	354	1667
	822	2264	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.64 (B) (INPUT = 0.90)
JSI METAL= 0.20 (G) (INPUT = 1.00)



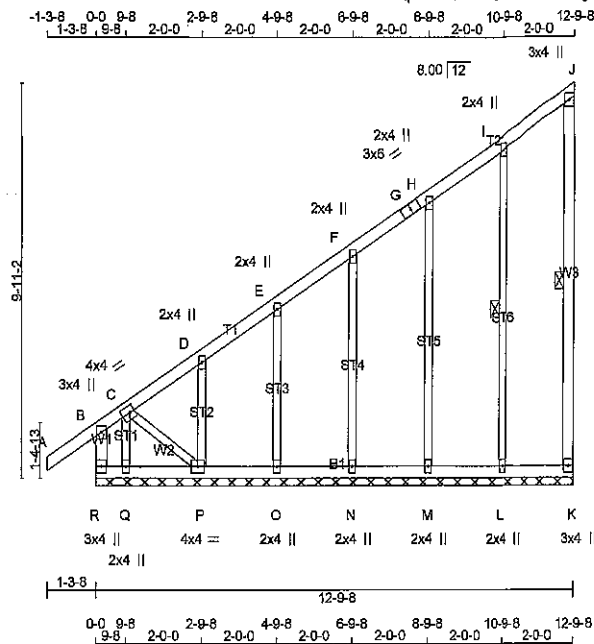
DRWG NO. TAM 5106-18
STRUCTURAL
COMPONENT ONLY

JOB NAME 286184	TRUSS NAME G27A	QUANTITY 2	PLY 1	JOB DESC. 43954 TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1:58.2

TOTAL WEIGHT = 2 X 71 = 141 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - G	2x4	DRY	No.2
G - J	2x4	DRY	No.2
K - J	2x4	DRY	No.2
R - B	2x4	DRY	No.2
R - K	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			
GABLE STUDS SPACED AT 2'-0" OC.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.
1 LATERAL BRACE(S) AT 1/2 LENGTH OF J-K, I-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0/32	-84.3 -84.3	0.11 (1)	10.00	L-I	-189/0	0.09 (1)
B-C	-48/0	-84.3 -84.3	0.11 (1)	6.25	M-H	-164/0	0.15 (1)
C-D	-12/0	-84.3 -84.3	0.04 (1)	6.25	N-F	-167/0	0.09 (1)
D-E	-12/0	-84.3 -84.3	0.04 (1)	6.25	O-E	-165/0	0.05 (1)
E-F	-7/0	-84.3 -84.3	0.04 (1)	10.00	P-D	-173/0	0.03 (1)
F-G	-5/0	-84.3 -84.3	0.04 (1)	10.00	Q-C	-39/0	0.01 (1)
G-H	-5/0	-84.3 -84.3	0.04 (1)	10.00	C-P	0/18	0.00 (1)
H-I	0/0	-84.3 -84.3	0.04 (1)	10.00			
I-J	-9/0	-84.3 -84.3	0.04 (1)	10.00			
K-J	-72/0	0.0 0.0	0.03 (1)	6.25			
R-B	-236/0	0.0 0.0	0.02 (1)	7.81			
R-Q	0/0	-28.0 -28.0	0.02 (2)	10.00			
Q-P	0/0	-28.0 -28.0	0.02 (2)	10.00			
P-O	0/9	-28.0 -28.0	0.02 (2)	10.00			
O-N	0/6	-28.0 -28.0	0.02 (2)	10.00			
N-M	0/4	-28.0 -28.0	0.02 (2)	10.00			
M-L	0/2	-28.0 -28.0	0.03 (2)	10.00			
L-K	0/0	-28.0 -28.0	0.03 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11 (A-B:1), BC=0.03 (L-M:2), WB=0.15 (H-M:1), SS=0.08 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

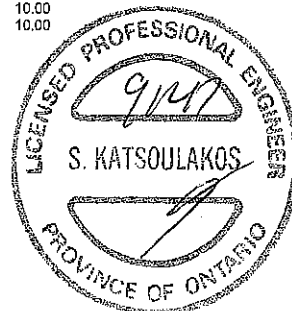
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1657 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.13 (I) (INPUT = 0.90)
JSI METAL= 0.05 (I) (INPUT = 1.00)



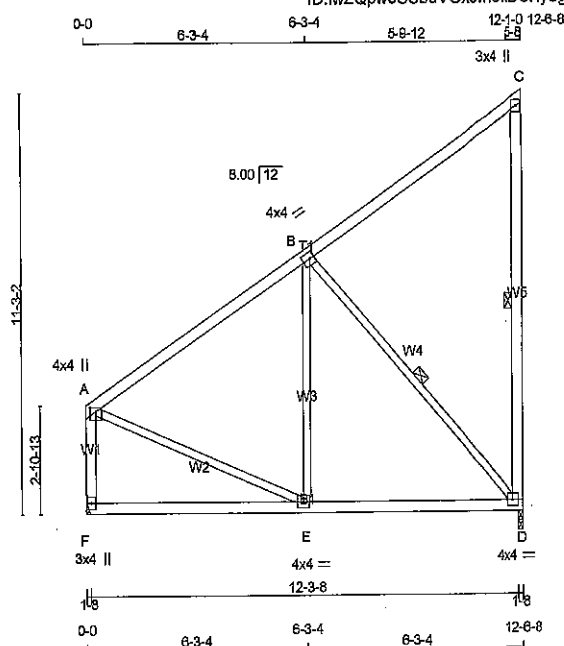
DRWNG. PAN 46206 17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
286185	T28A	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.200 S Dec 12 2017 MiTek Industries, Inc. Tue Jan 23 08:32:14 2018 Page 1

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Scale = 1:61.9

TOTAL WEIGHT = 2 X 64 = 128 lb

LUMBER

N. L. G. A. RULES					
CHORDS		SIZE	LUMBER		DESCR
A - C	2x4	DRY	No.2		SPF
D - C	2x4	DRY	No.2		SPF
F - A	2x4	DRY	No.2		SPF
F - D	2x4	DRY	No.2		SPF
ALL WEBS	2x3	DRY	No.2		SPF
EXCEPT					

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00
B	TMWW-t	MT20	4.0	4.0	2.00	1.50
C	TMV+p	MT20	3.0	4.0		
D	BMVW1-t	MT20	4.0	4.0		
E	BMWW-t	MT20	4.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
D	704	0	0	1-8
F	704	0	0	1-8

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
D	578	321 / 0	132 / 0	0 / 0	0 / 0	125 / 0	0 / 0
F	578	321 / 0	132 / 0	0 / 0	0 / 0	125 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-D, B-D.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (UNBRACED LENGTH)	FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)
A-B		-452 / 0	-84.3	-84.3	0.44 (1)	8.25	E-B		0 / 299	0.07 (3)
B-C		-36 / 0	-84.3	-84.3	0.43 (1)	6.25	B-D		-598 / 0	0.34 (1)
D-C		-199 / 0	0.0	0.0	0.13 (1)	8.25	A-E		0 / 440	0.10 (1)
F-A		-637 / 0	0.0	0.0	0.10 (1)	7.81				
F-E		0 / 0	-28.0	-28.0	0.32 (3)	10.00				
E-D		0 / 407	-28.0	-28.0	0.37 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.42")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.42")
CALCULATED VERT. DEFL.(TL) = L/999 (0.08")

CSI: TC=0.44/1.00 (A-B:1), BC=0.37/1.00 (D-E:2), WB=0.34/1.00 (B-D:1), SSI=0.21/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	822	2284	1656

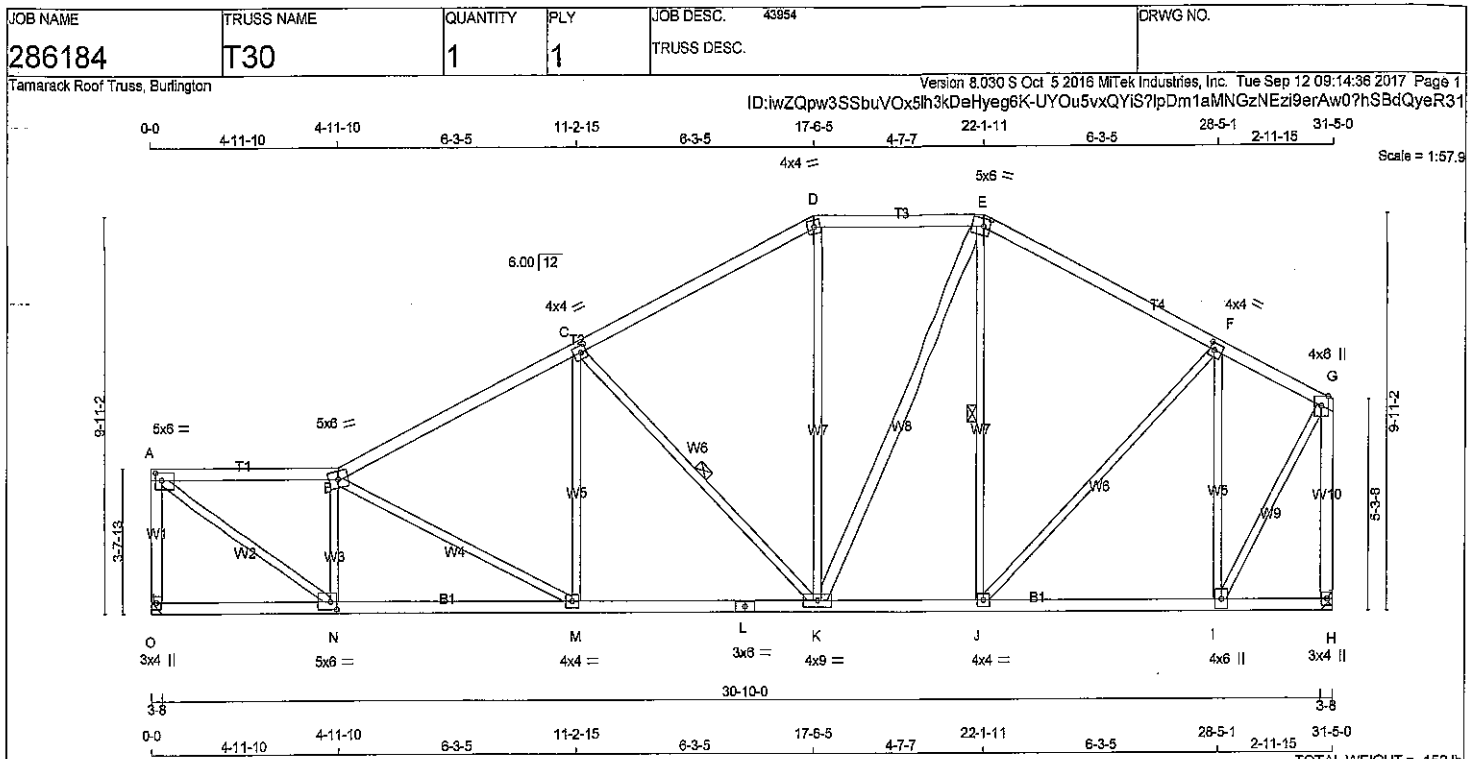
PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.50 (E) (INPUT = 0.90)
JSI METAL= 0.15 (E) (INPUT = 1.00)



DWG NO. TAM 5109-18
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 15210

[M/F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER
O - A	2x4 DRY	No.2
A - B	2x4 DRY	No.2
B - D	2x4 DRY	No.2
D - E	2x4 DRY	No.2
E - G	2x4 DRY	No.2
H - G	2x4 DRY	No.2
O - L	2x4 DRY	No.2
L - H	2x4 DRY	No.2

ALL WEBS EXCEPT	2x3 DRY	No.2
K - E	2x4 DRY	No.2

DRY: SEASONED LUMBER.

DESCR.

SPF
SPF
SPF
SPF
SPF
SPF
SPF
SPF

SPF

SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG			
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
O	1763	0	1763	0	0		
H	1763	0	1763	0	0		

HANGER BY OTHERS
MIN. SEAT SIZE: 3-8
HANGER BY OTHERS
MIN. SEAT SIZE: 3-8

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1448	804 / 0	330 / 0	0 / 0	0 / 0	314 / 0	0 / 0
H	1448	804 / 0	330 / 0	0 / 0	0 / 0	314 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.02 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-K, E-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
O-A	-1707 / 0	0.0	0.0 0.35 (1)	A-N	0 / 2643	0.59 (1)	
A-B	-2175 / 0	-84.3	-84.3 0.33 (1)	N-B	-1339 / 0	0.29 (1)	
B-C	-2253 / 0	-84.3	-84.3 0.54 (1)	B-M	-197 / 0	0.20 (1)	
C-D	-1615 / 0	-84.3	-84.3 0.48 (1)	M-C	0 / 413	0.09 (2)	
D-E	-1421 / 0	-84.3	-84.3 0.26 (1)	C-K	-889 / 0	0.51 (1)	
E-F	-1373 / 0	-84.3	-84.3 0.43 (1)	K-D	0 / 358	0.08 (2)	
F-G	-844 / 0	-84.3	-84.3 0.32 (1)	K-E	0 / 509	0.08 (1)	
H-G	-1746 / 0	0.0	0.0 0.60 (1)	J-F	-266 / 42	0.18 (1)	
				J-F	0 / 599	0.13 (1)	
				I-F	-1186 / 0	0.91 (1)	
O-N	0 / 0	-28.0	-28.0 0.19 (3)	I-G	0 / 1543	0.35 (1)	
N-M	0 / 2216	-28.0	-28.0 0.51 (2)				
M-L	0 / 2042	-28.0	-28.0 0.48 (2)				
L-K	0 / 2042	-28.0	-28.0 0.48 (2)				
K-J	0 / 1205	-28.0	-28.0 0.32 (2)				
J-I	0 / 795	-28.0	-28.0 0.28 (2)				
I-H	0 / 0	-28.0	-28.0 0.19 (3)				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0	2.25	2.00
B	TTWW-m	MT20	5.0	6.0		
C	TMVW-t	MT20	4.0	4.0	2.00	1.75
D	TTW-m	MT20	4.0	4.0		
E	TTWW-m	MT20	5.0	6.0	2.25	2.00
F	TMVW-t	MT20	4.0	4.0	2.00	1.50
G	TMVW-p	MT20	4.0	6.0	Edge	
H	BMV-t+p	MT20	3.0	4.0		
I	BMVW-t	MT20	4.0	6.0		
J	BMVW-t	MT20	4.0	4.0		
K	BMVW-t	MT20	4.0	9.0		
L	BS-t	MT20	3.0	6.0		
M	BMVW-t	MT20	4.0	4.0		
N	BMVW-t	MT20	5.0	6.0	2.25	2.00
O	BMV-t+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	OL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	OL	=	7.0	PSF
TOTAL LOAD		=	48.1	PSF

SPACING = 24.0 IN./C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-06
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.05")
CALCULATED VERT. DEFL.(LL)= L/999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (1.05")
CALCULATED VERT. DEFL.(TL)= L/999 (0.22")

CSI: TC=0.90 (G-H-1), BC=0.51 (M-N-2),
WB=0.91 (F-I-1), SSI=0.23 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1967 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

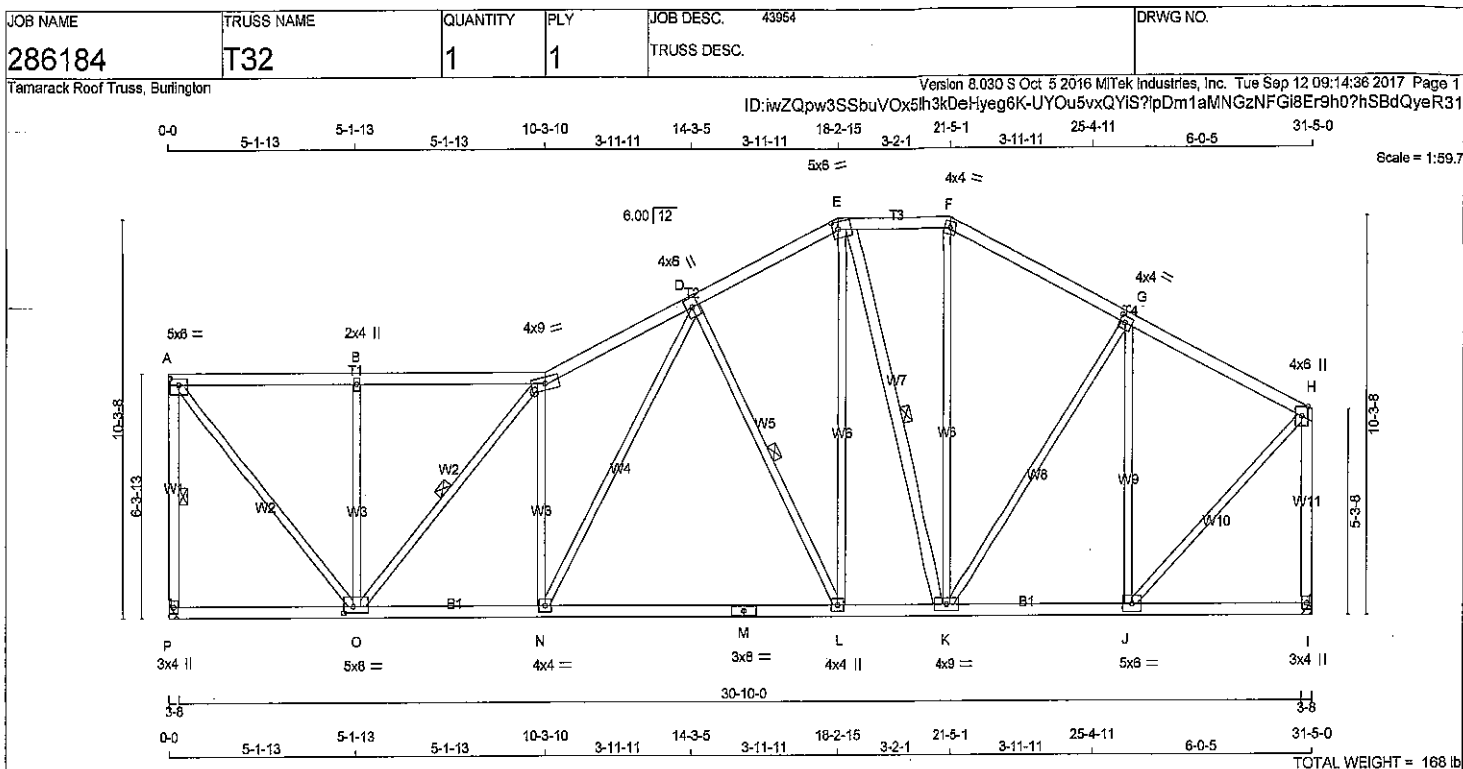
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)

JSI METAL= 0.61 (L) (INPUT = 1.00)

DWG NO. TAM46187-17

STRUCTURAL
COMPONENT ONLY



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
P - A	2x4	DRY	No.2
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
I - H	2x4	DRY	No.2
P - M	2x4	DRY	No.2
M - I	2x4	DRY	No.2
ALL WEBS EXCEPT E - K	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0	2.00	3.00
B	TMVW-w	MT20	2.0	4.0		
C	TTWW-m	MT20	4.0	9.0		
D	TMVW-t	MT20	4.0	6.0		
E	TTWW-m	MT20	5.0	8.0	2.25	1.75
F	TTW-m	MT20	4.0	4.0		
G	TMVW-t	MT20	4.0	4.0	2.00	1.75
H	TMVW-w	MT20	4.0	6.0	Edge	
I	BMV-t	MT20	3.0	4.0		
J	BMVW-t	MT20	5.0	6.0		
K	BMVW-w	MT20	4.0	9.0		
L	BMVW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	8.0		
N	BMVW-t	MT20	4.0	4.0		
O	BMVW-w	MT20	5.0	8.0	2.00	3.00
P	BMV-t	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
P	1763	0	1763	0	HANGER BY OTHERS	MIN. SEAT SIZE: 3-8
I	1763	0	1763	0	HANGER BY OTHERS	MIN. SEAT SIZE: 3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	WIND			
P	1448	804 / 0	330 / 0	0 / 0	0 / 0	314 / 0	0 / 0
I	1448	804 / 0	330 / 0	0 / 0	0 / 0	314 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.34 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF A-P, C-O, D-L, E-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

CHORDS					WEBS				
MAX. FACTORED		FACTORED			MAX. FACTORED				
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CS1 (LC)	MAX. UNBRACED LENGTH	MEMB.	FORCE (LBS)	MAX CS1 (LC)		
FR-TO		FROM TO			FR-TO				
P-A	-1700 / 0	0.0	0.0	0.32 (1)	5.10	A-O	0 / 2023	0.46 (1)	
A-B	-1320 / 0	-84.3	-84.3	0.30 (1)	5.29	O-B	-535 / 0	0.34 (1)	
B-C	-1320 / 0	-84.3	-84.3	0.30 (1)	5.29	C-O	-1126 / 0	0.46 (1)	
C-D	-2260 / 0	-84.3	-84.3	0.23 (1)	4.34	N-C	-459 / 0	0.29 (1)	
D-E	-1532 / 0	-84.3	-84.3	0.19 (1)	5.13	D-L	-613 / 0	0.45 (1)	
E-F	-1211 / 0	-84.3	-84.3	0.12 (1)	5.69	L-E	0 / 912	0.21 (1)	
F-G	-1374 / 0	-84.3	-84.3	0.29 (1)	5.22	E-K	-517 / 0	0.30 (1)	
G-H	-1137 / 0	-84.3	-84.3	0.28 (1)	5.63	K-F	0 / 328	0.07 (2)	
H-I	-1708 / 0	0.0	0.0	0.88 (1)	6.36	K-G	0 / 305	0.07 (1)	
						J-G	-862 / 0	0.99 (1)	
P-O	0 / 0	-28.0	-28.0	0.16 (2)	10.00	J-H	0 / 1480	0.33 (1)	
O-N	0 / 2049	-28.0	-28.0	0.53 (2)	10.00	N-D	0 / 735	0.17 (1)	
N-M	0 / 1724	-28.0	-28.0	0.50 (2)	10.00				
M-L	0 / 1724	-28.0	-28.0	0.50 (2)	10.00				
L-K	0 / 1363	-28.0	-28.0	0.46 (2)	10.00				
K-J	0 / 1037	-28.0	-28.0	0.31 (2)	10.00				
J-I	0 / 0	-28.0	-28.0	0.19 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, ECBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.05")
CALCULATED VERT. DEFL.(LL) = L/999 (0.19")
ALLOWABLE DEFL.(TL) = L/360 (1.05")
CALCULATED VERT. DEFL.(TL) = L/999 (0.31")

CSI: TC=0.88 (H-I), BC=0.53 (N-O), WB=0.99 (G-J), SSI=0.21 (A-B)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

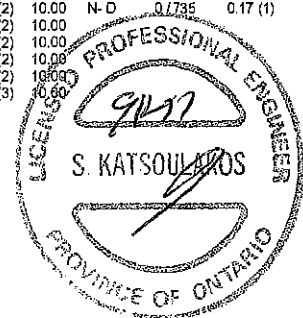
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 622 2284 1656

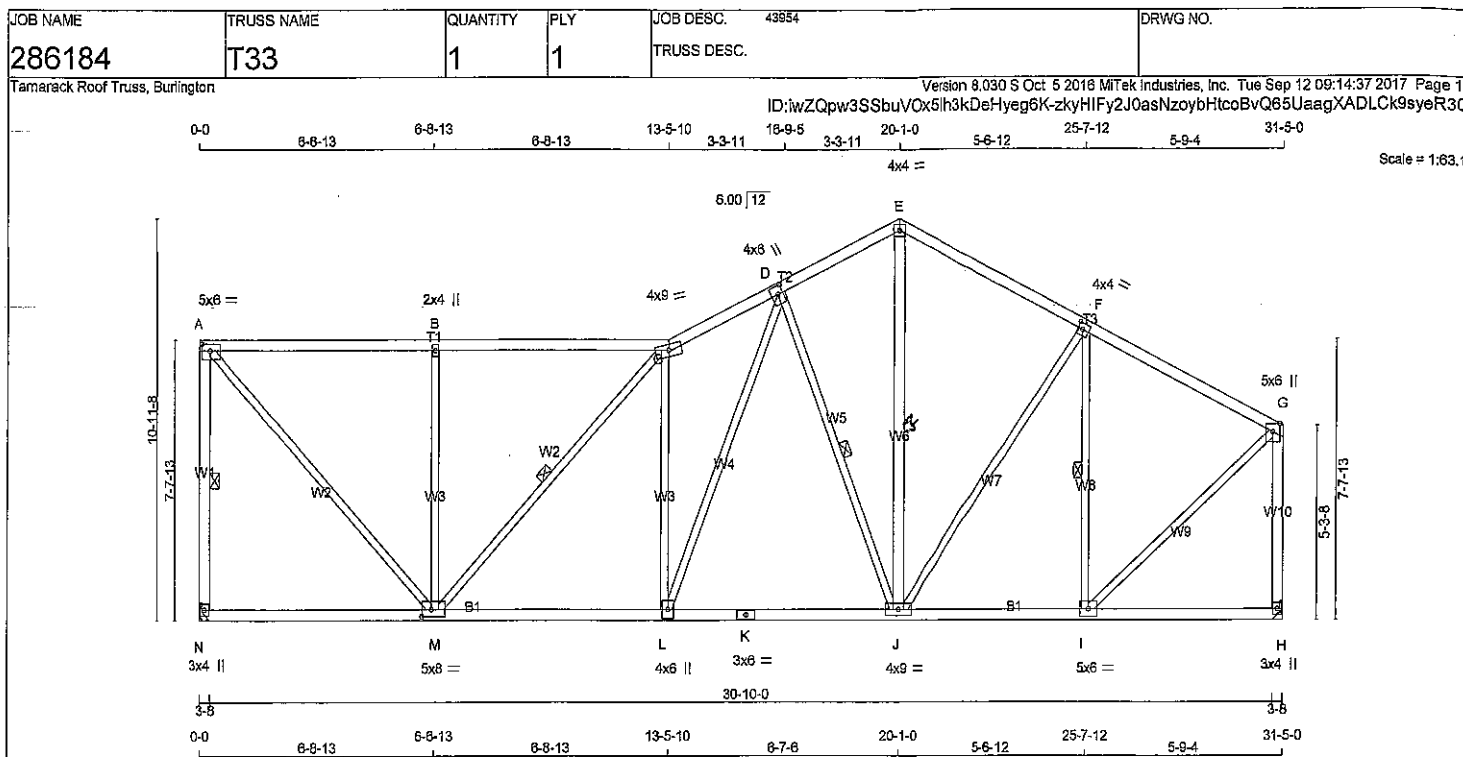
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (H) (INPUT = 0.90)
JSI METAL= 0.67 (C) (INPUT = 1.00)



DWG NO. TAN 46189-17
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS

SIZE

LUMBER

DESCR.

A - A

2x4

DRY

No.2

SPF

A - C

2x4

DRY

No.2

SPF

C - E

2x4

DRY

No.2

SPF

E - G

2x4

DRY

No.2

SPF

H - G

2x4

DRY

No.2

SPF

N - K

2x4

DRY

No.2

SPF

K - H

2x4

DRY

No.2

SPF

ALL WEBS

2x3

DRY

No.2

SPF

EXCEPT

J - E

2x4

DRY

No.2

SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT

TYPE

PLATES

W

LEN

Y

X

A

TMVW-t

MT20

5.0

6.0

2.25

2.75

B

TMVW-w

MT20

2.0

4.0

C

TTWW-m

MT20

4.0

9.0

D

TMVW-t

MT20

4.0

6.0

2.75

1.75

E

TTW-p

MT20

4.0

4.0

F

TMVW-t

MT20

4.0

4.0

2.00

1.75

G

TMVW-p

MT20

5.0

6.0

Edge

H

BMV1-p

MT20

3.0

4.0

I

BMVW-t

MT20

5.0

6.0

J

BMVW-w

MT20

4.0

9.0

K

BS-t

MT20

3.0

6.0

L

BMVW-t

MT20

4.0

6.0

M

BMVW-w

MT20

5.0

6.0

2.25

3.25

N

BMV1-p

MT20

3.0

4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED

GROSS REACTION

JT

VERT

HORZ

N

1763

0

MAXIMUM FACTORED

GROSS REACTION

DOWN

HORZ

UPLIFT

1763

0

0

INPUT

REQD

BRG

BRG

IN-SX

IN-SX

HANGER BY OTHERS

MIN. SEAT SIZE: 3-8

HANGER BY OTHERS

MIN. SEAT SIZE: 3-8

UNFACTORED REACTIONS

1ST LCASE

MAX./MIN. COMPONENT REACTIONS

JT

COMBINED

SNOW

LIVE

PERM. LIVE

WIND

DEAD

SOIL

N

1448

804 / 0

330 / 0

0 / 0

0 / 0

314 / 0

0 / 0

H

1448

804 / 0

330 / 0

0 / 0

0 / 0

314 / 0

0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.58 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF A-N, C-M, D-J, F-I. *E-S*

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB.

MAX. FACTORED FORCE (LBS)

FACTORED VERT. LOAD (PLF)

MAX. CS1 (LC)

FR-TO

N-A

-1686 / 0

0.0

0.0 0.44 (1)

A-B

-1353 / 0

-84.3

-84.3 0.53 (1)

B-C

-1353 / 0

-84.3

-84.3 0.52 (1)

C-D

-2070 / 0

-84.3

-84.3 0.17 (1)

D-E

-1365 / 0

-84.3

-84.3 0.14 (1)

E-F

-1377 / 0

-84.3

-84.3 0.38 (1)

F-G

-1213 / 0

-84.3

-84.3 0.37 (1)

H-G

-1697 / 0

0.0

0.0 0.87 (1)

N-M

0 / 0

-28.0

-28.0 0.31 (3)

M-L

0 / 1659

-28.0

-28.0 0.52 (2)

L-K

0 / 1559

-28.0

-28.0 0.41 (2)

K-J

0 / 1559

-28.0

-28.0 0.41 (2)

J-I

0 / 1108

-28.0

-28.0 0.34 (2)

I-H

0 / 0

-28.0

-28.0 0.21 (3)

WEBS

MEMB.

MAX. FACTORED FORCE (LBS)

MAX. CS1 (LC)

FR-TO

A-M

0 / 1097

0.45 (1)

M-B

-704 / 0

0.76 (1)

M-C

-751 / 0

0.52 (1)

L-C

-681 / 0

0.73 (1)

D-J

-951 / 0

0.62 (1)

J-E

0 / 918

0.15 (1)

J-F

0 / 173

0.04 (1)

I-F

-785 / 0

0.33 (1)

I-G

0 / 1464

0.33 (1)

L-D

0 / 315

0.21 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.8 PSF

DL = 3.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN./C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014

- CSA 086-09

- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.05")

CALCULATED VERT. DEFL.(LL)= L/999 (0.12")

ALLOWABLE DEFL.(TL)= L/360 (1.05")

CALCULATED VERT. DEFL.(TL)= L/999 (0.21")

CSI: TC=0.87 (G-H:1), BC=0.52 (L-M:2), WB=0.76 (B-M:1), SSI=0.28 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE

GRIP(DRY)

SHEAR

SECTION

(PSI)

(PLI)

(PLI)

MAX MIN

MAX MIN

MAX MIN

MT20

618

354

1667

822

2254

1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (A) (INPUT = 0.90)

JSI METAL= 0.53 (C) (INPUT = 1.00)

9117

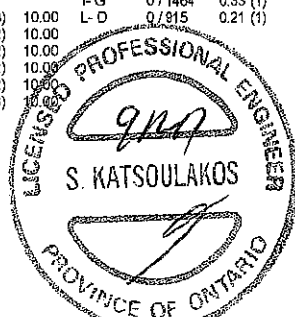
S. KATSOUKAKOS

PROVINCE OF ONTARIO

DWG NO. TAN 46190-17

STRUCTURAL

COMPONENT ONLY

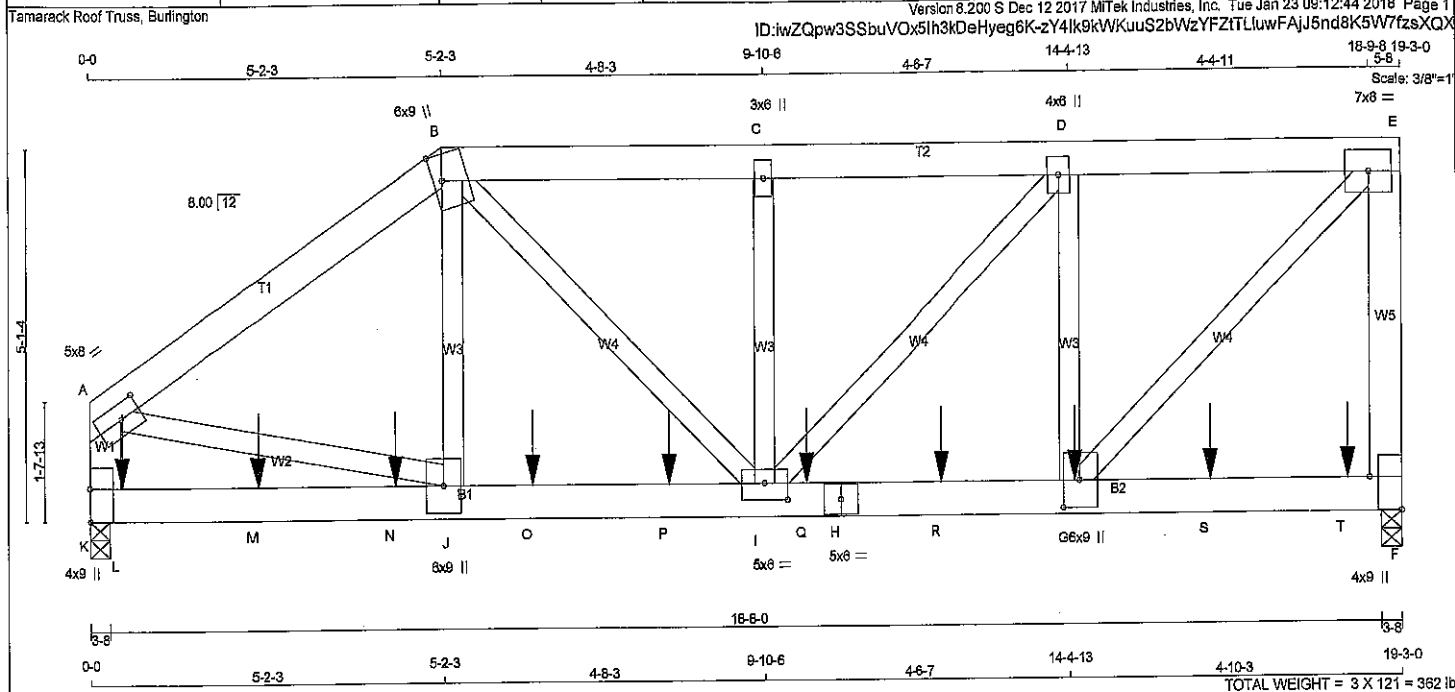


DWG NO. TAN 46190-17
STRUCTURAL
COMPONENT ONLY



ISI GRIP= 0.90 (I) (INPUT = 0.90)
ISI METAL= 0.59 (J) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286186	T36	1	3	TRUSS DESC.	



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x6	DRY	No.2	SPF
B - E	2x6	DRY	No.2	SPF
F - E	2x6	DRY	No.2	SPF
K - A	2x6	DRY	No.2	SPF
K - H	2x6	DRY	2100F 1.8E	SPF
H - F	2x6	DRY	2100F 1.8E	SPF

ALL WEBS 2x4 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - B 2 12	TOP	
B - E 2 12	TOP	
E - F 2 12	TOP	
K - A 2 12	TOP	
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
K - H 2 6	SIDE(494.5)	
H - F 2 8	SIDE(579.7)	
WEBS : (0.122"x3") SPIRAL NAILS		
2x4 1 6		

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	8.0	2.50 3.75
B	TTWW+m	MT20	6.0	9.0	4.25 1.50
C	TMVW+w	MT20	3.0	6.0	
D	TMVW+t	MT20	4.0	6.0	
E	TMVW-t	MT20	7.0	8.0	
F	BMV1+p	MT20	4.0	9.0	5.50 Edge
G	BMVW+t	MT20	6.0	9.0	4.50 2.75
H	BS-t	MT20	5.0	6.0	
I	BMVW-w	MT20	5.0	8.0	2.75 4.00
J	BMVW+t	MT20	6.0	9.0	
K	BMV1+p	MT20	4.0	9.0	Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ
JT	8622	0	8622	0
F	8622	0	8622	0
K	9903	0	9903	0

UNFACTORED REACTIONS

	1ST CASE	MAX	MIN	COMPONENT REACTIONS
	COMBINED	SNOW	LIVE	PERM.LIVE
JT	7842	4447	1724	0
F	7842	4447	1724	0
K	8073	4576	1777	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.80 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	MAX. FACTORED UNBRACED LENGTH (FT)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)
MEMB.						MEMB.		
FR-TO						FR-TO		
A-B	-9882 / 0	-84.3	-84.3	0.12 (1)	4.61	J-B	0 / 3368	0.18 (1)
B-C	-10010 / 0	-84.3	-84.3	0.11 (1)	4.60	G-E	0 / 10826	0.58 (1)
C-D	-10010 / 0	-84.3	-84.3	0.10 (1)	4.60	B-I	0 / 2387	0.13 (1)
D-E	-7818 / 0	-84.3	-84.3	0.09 (1)	5.09	G-D	-2638 / 0	0.21 (1)
F-E	-7694 / 0	0.0	0.0	0.61 (1)	6.48	I-C	-342 / 0	0.03 (1)
K-A	-7491 / 0	0.0	0.0	0.16 (1)	6.55	I-D	0 / 3138	0.17 (1)
						A-J	0 / 8393	0.45 (1)
K-L	0 / 0	-28.0	-28.0	0.29 (1)	10.00			
L-M	0 / 0	-28.0	-28.0	0.29 (1)	10.00			
M-N	0 / 0	-28.0	-28.0	0.29 (1)	10.00			
N-J	0 / 0	-28.0	-28.0	0.29 (1)	10.00			
J-O	0 / 8311	-28.0	-28.0	0.42 (1)	10.00			
O-P	0 / 8311	-28.0	-28.0	0.42 (1)	10.00			
P-I	0 / 8311	-28.0	-28.0	0.42 (1)	10.00			
I-Q	0 / 7818	-28.0	-28.0	0.33 (1)	10.00			
Q-H	0 / 7818	-28.0	-28.0	0.33 (1)	10.00			
H-R	0 / 7818	-28.0	-28.0	0.33 (1)	10.00			
R-G	0 / 7818	-28.0	-28.0	0.33 (1)	10.00			
G-S	0 / 0	-28.0	-28.0	0.21 (1)	10.00			
S-T	0 / 0	-28.0	-28.0	0.21 (1)	10.00			
T-F	0 / 0	-28.0	-28.0	0.21 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX	FACE
G	14-5-12	-1735	-1735	BACK
L	5-12	-1743	-1743	BACK
M	2-5-12	-1735	-1735	BACK
N	4-5-12	-1735	-1735	BACK
O	6-5-12	-1735	-1735	BACK
P	8-5-12	-1735	-1735	BACK
Q	10-5-12	-1735	-1735	BACK
R	12-5-12	-1735	-1735	BACK
S	16-5-12	-1735	-1735	BACK
T	18-5-12	-1739	-1739	BACK

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/380 (0.64")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (0.64")
CALCULATED VERT. DEFL.(TL) = L/999 (0.12")

CSI: TC=0.61/1.00 (E-F-1), BC=0.42/1.00 (J-K-1), WB=0.58/1.00 (E-G-1), SSI=0.58/1.00 (J-K-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
	MAX	MIN
MT20	618	354
	1657	822
	2284	1655

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.99 (J) (INPUT = 0.90)
JSI METAL= 0.82 (A) (INPUT = 1.00)

DRWG NO. TAM5097 -18
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286186	T36	1	3	TRUSS DESC.	

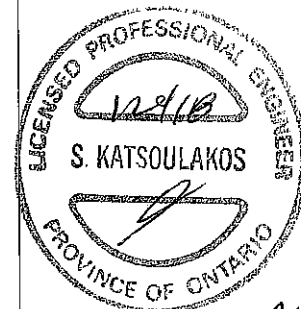
Tamarack Roof Truss, Burlington

Version 8.200 S Dec 12 2017 Mitek Industries, Inc. Tue Jan 23 09:12:44 2018 Page 2

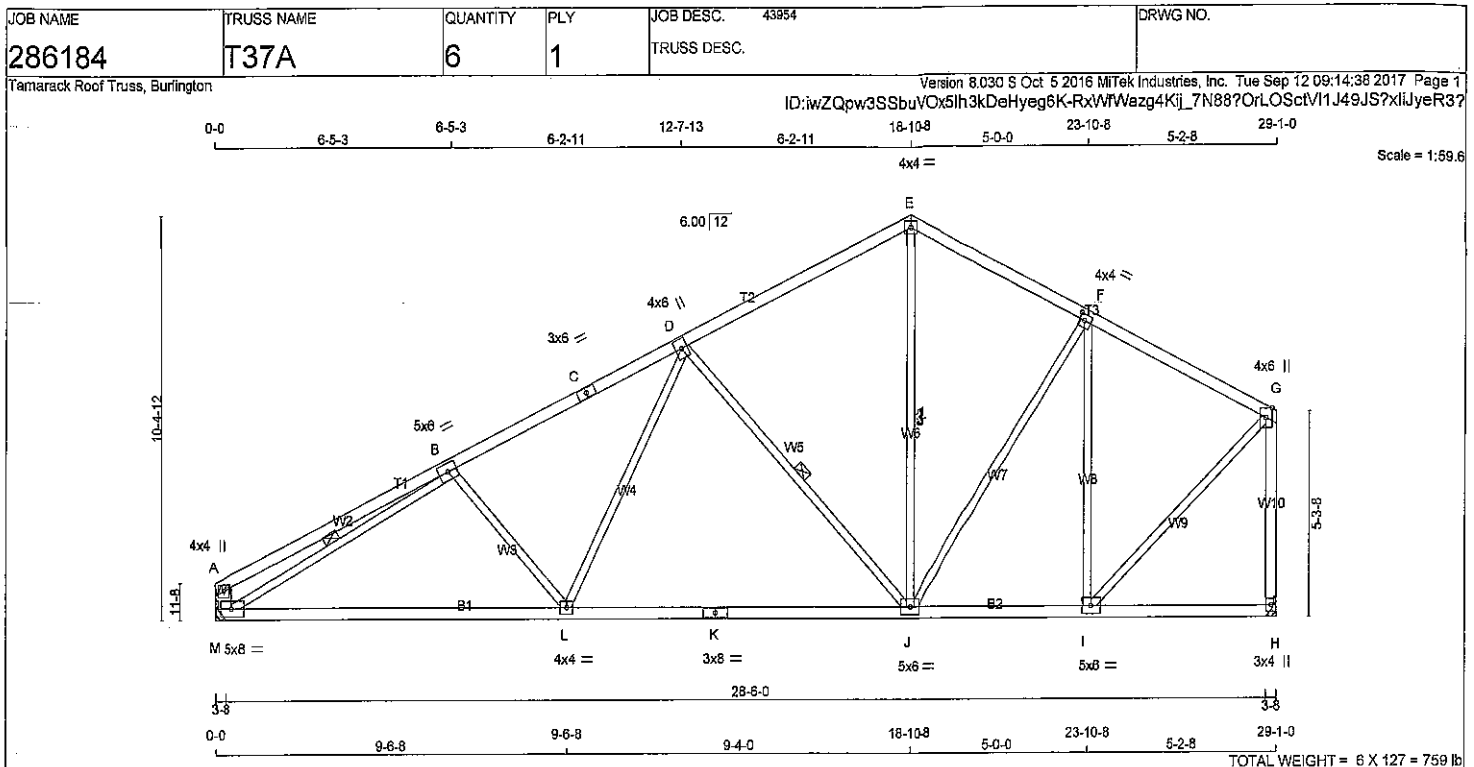
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HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1742.6 lbs FACTORED DOWN AT 5-12, 1735.3 lbs FACTORED DOWN AT 2-5-12, 1735.3 lbs FACTORED DOWN AT 4-5-12, 1735.3 lbs FACTORED DOWN AT 6-5-12, 1735.3 lbs FACTORED DOWN AT 8-5-12, 1735.3 lbs FACTORED DOWN AT 10-5-12, 1735.3 lbs FACTORED DOWN AT 12-5-12, 1735.3 lbs FACTORED DOWN AT 14-5-12, AND 1735.3 lbs FACTORED DOWN AT 16-5-12, AND 1738.5 lbs FACTORED DOWN AT 18-5-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.



DWG NO. TAM 5097-18
STRUCTURAL
COMPONENT ONLY



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
H - G	2x4	DRY	No.2
M - A	2x6	DRY	No.2
M - K	2x4	DRY	No.2
K - H	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	4.0	4.0		
B	TMWW-t	MT20	5.0	6.0		
C	TS-t	MT20	3.0	6.0		
D	TMWW+t	MT20	4.0	6.0		
E	TTW-p	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0	2.00	1.75
G	TMVW+p	MT20	4.0	6.0	Edge	
H	BMVt+p	MT20	3.0	4.0		
I	BMWW-t	MT20	5.0	6.0		
J	BMWW-t	MT20	5.0	6.0		
K	BS-t	MT20	3.0	6.0		
L	BMVW-t	MT20	4.0	4.0		
M	BMVW-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
H	1632	0	1632	0	0	IN-SX
M	1632	0	1632	0	0	IN-SX

UNFACTORED REACTIONS							
	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1340	744 / 0	305 / 0	0 / 0	0 / 0	291 / 0	0 / 0
M	1340	744 / 0	305 / 0	0 / 0	0 / 0	291 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.19 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-J, B-M. *e-s*

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MAX. FACTORED		FACTORED		MAX. UNBRACED LENGTH	MAX. FACTORED		MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)		MEMB.	FORCE (LBS)			
FR-TO		FROM	TO		FR-TO				
A-B	0 / 22	-84.3	-84.3	0.43 (1)	10.00	B-L	-329 / 52	0.14 (1)	
B-C	-2191 / 0	-84.3	-84.3	0.46 (1)	4.19	L-D	0 / 643	0.14 (2)	
C-D	-2191 / 0	-84.3	-84.3	0.46 (1)	4.19	D-J	-942 / 0	0.58 (1)	
D-E	-1236 / 0	-84.3	-84.3	0.41 (1)	5.27	J-E	0 / 717	0.16 (1)	
E-F	-1236 / 0	-84.3	-84.3	0.30 (1)	5.43	J-F	0 / 224	0.05 (1)	
F-G	-1053 / 0	-84.3	-84.3	0.29 (1)	5.78	I-F	-782 / 0	0.92 (1)	
G-H	-1570 / 0	0.0	0.0	0.81 (1)	6.58	I-G	0 / 1331	0.30 (1)	
H-A	-217 / 0	0.0	0.0	0.01 (1)	7.81	M-B	-2536 / 0	0.89 (1)	
M-L	0 / 2172	-28.0	-28.0	0.89 (2)	10.00				
L-K	0 / 1712	-28.0	-28.0	0.85 (2)	10.00				
K-J	0 / 1712	-28.0	-28.0	0.85 (2)	10.00				
J-I	0 / 962	-28.0	-28.0	0.42 (2)	10.00				
I-H	0 / 0	-28.0	-28.0	0.16 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. G/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/999 (0.31")
ALLOWABLE DEFL.(TL) = L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/688 (0.51")

CSI: TC=0.81 (G-H:1), BC=0.89 (L-M:2), WB=0.92 (F-I:1), SSI=0.22 (L-M:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HELLS OFF

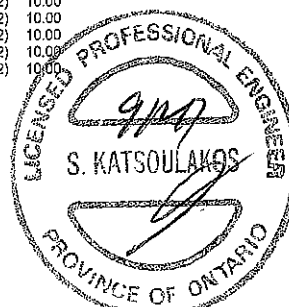
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1655

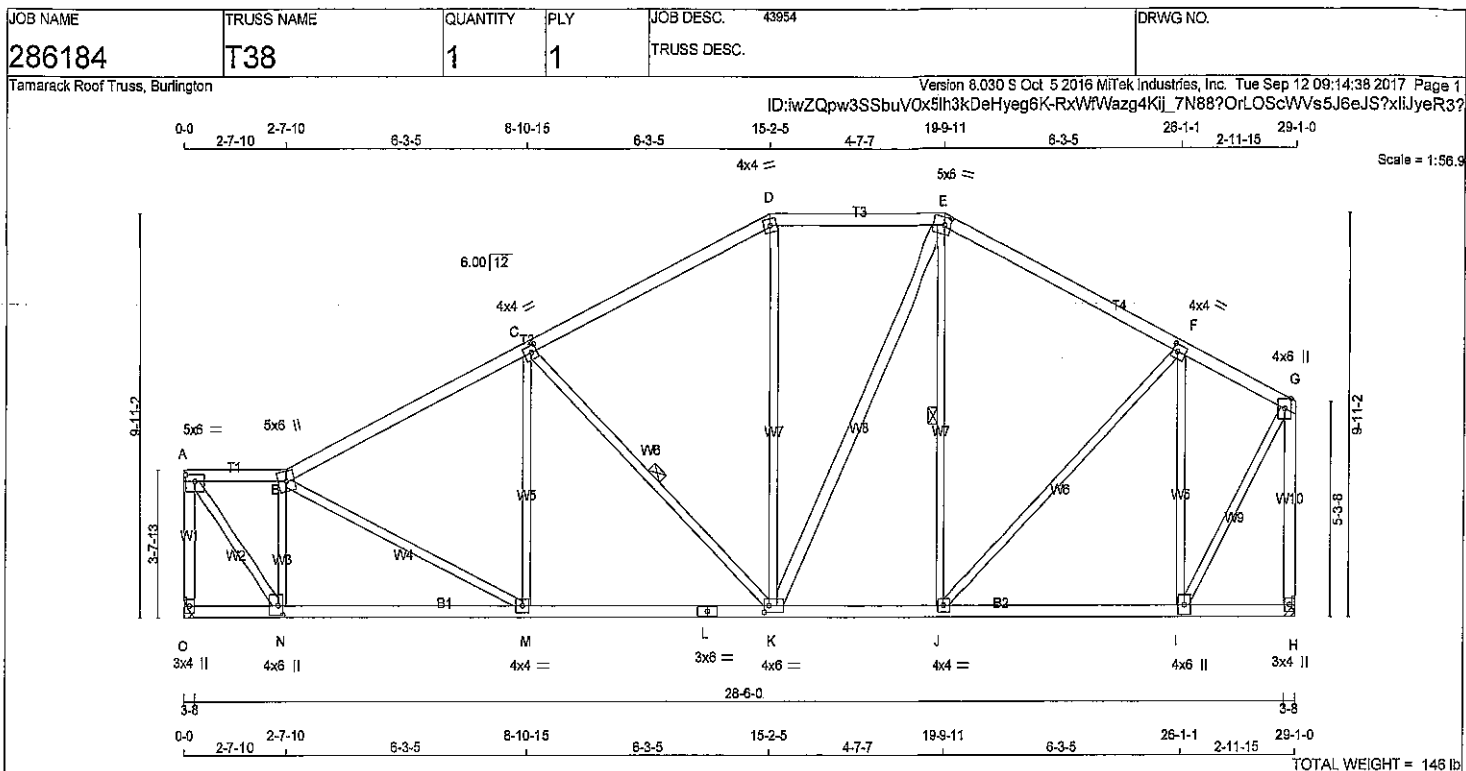
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.87 (K) (INPUT = 1.00)



DWG NO. TAM 46193-17
STRUCTURAL
COMPONENT ONLY



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
O - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT K - E	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVV-t	MT20	5.0	6.0	2.00	3.00
B	TTWW+m	MT20	5.0	6.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTW-m	MT20	4.0	4.0		
E	TTWW-m	MT20	5.0	6.0	2.25	2.00
F	TMWW-t	MT20	4.0	4.0	2.00	1.50
G	TMWW+p	MT20	4.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWWH-t	MT20	4.0	6.0	2.00	1.50
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMWW-t	MT20	4.0	6.0	2.75	1.50
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
O	1632	0	1632	0	0	0
H	1632	0	1632	0	0	0

MIN. SEAT SIZE: 3-8
HANGER BY OTHERS

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
O	1340	744 / 0	305 / 0	0 / 0	0 / 0	291 / 0	0 / 0
H	1340	744 / 0	305 / 0	0 / 0	0 / 0	291 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.44 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

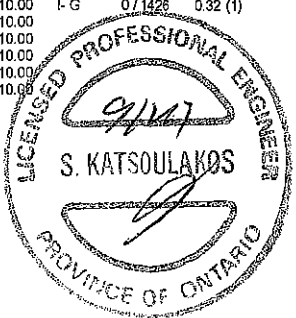
1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-K, E-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LOAD LC1	MAX. CS1 (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM	TO			FR-TO			
O-A	-1616 / 0	0.0	0.0	0.33 (1)	6.50	A-N	0 / 1891	0.43 (1)	
A-B	-1138 / 0	-84.3	-84.3	0.09 (1)	5.87	N-B	-1373 / 0	0.30 (1)	
B-C	-1796 / 0	-84.3	-84.3	0.50 (1)	4.44	B-M	0 / 515	0.12 (1)	
C-D	-1408 / 0	-84.3	-84.3	0.47 (1)	4.93	M-C	-50 / 229	0.05 (3)	
D-E	-1234 / 0	-84.3	-84.3	0.25 (1)	5.50	C-K	-575 / 0	0.33 (1)	
E-F	-1231 / 0	-84.3	-84.3	0.42 (1)	5.27	K-D	0 / 299	0.07 (2)	
F-G	-777 / 0	-84.3	-84.3	0.32 (1)	6.25	K-E	0 / 384	0.06 (1)	
H-G	-1615 / 0	0.0	0.0	0.83 (1)	6.51	J-E	-199 / 76	0.13 (1)	
						J-F	0 / 494	0.11 (1)	
O-N	0 / 0	-28.0	-28.0	0.15 (3)	10.00	I-F	-1066 / 0	0.83 (1)	
N-M	0 / 1181	-28.0	-28.0	0.39 (2)	10.00	I-G	0 / 1426	0.32 (1)	
M-L	0 / 1635	-28.0	-28.0	0.44 (2)	10.00				
L-K	0 / 1635	-28.0	-28.0	0.44 (2)	10.00				
K-J	0 / 1079	-28.0	-28.0	0.30 (2)	10.00				
J-I	0 / 735	-28.0	-28.0	0.27 (2)	10.00				
I-H	0 / 0	-28.0	-28.0	0.19 (3)	10.00				



DWG NO. TAM 46194-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/999 (0.09")
ALLOWABLE DEFL.(TL) = L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/999 (0.15")

CSI: TC=0.83 (G-H:1), BC=0.44 (K-M:2), WB=0.83 (F-I:1), SSI=0.23 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1658

PLATE PLACEMENT TOL. = 0.25D inches

PLATE ROTATION TOL. = 5.0 Deg.

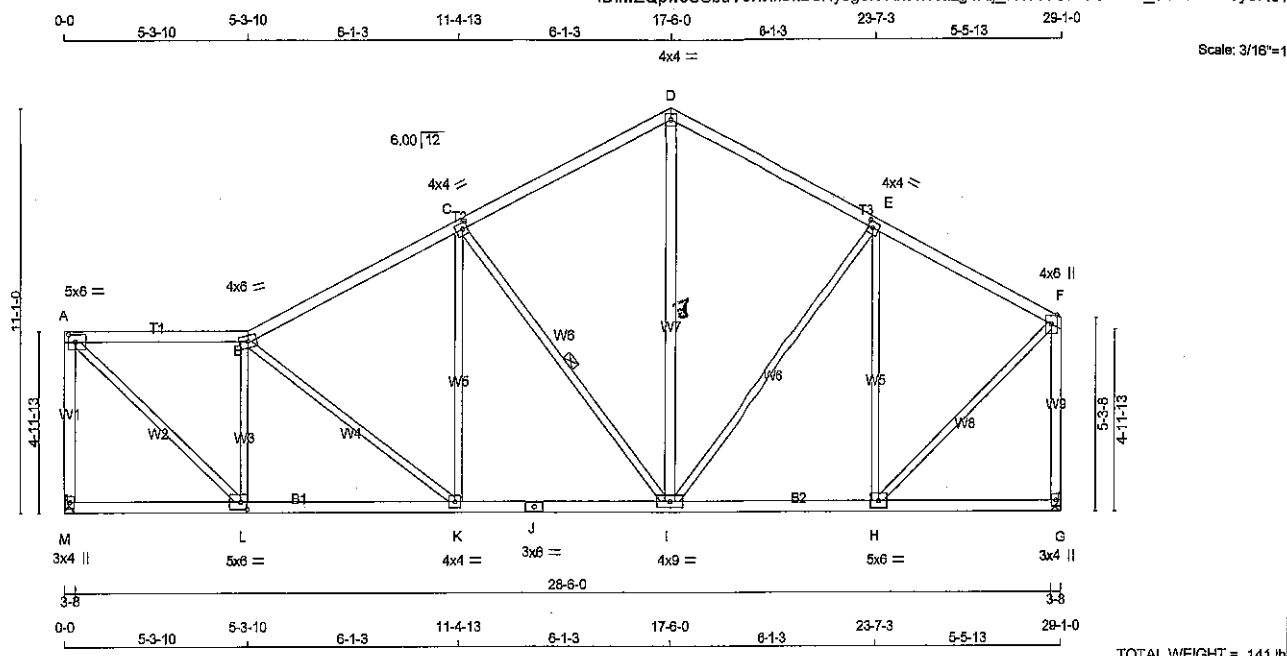
JSI GRIP=0.85 (A) (INPUT=0.90)
JSI METAL=0.49 (L) (INPUT=1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
286184	T39	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID: iwZQpw3SSbuV0x5In3kDeHyeg6K-RxWVWazg4Kij_7N88?OrLOScsVs_J4VJS?xliJyeR3?



TOTAL WEIGHT = 141 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE		
M - A	2x4	DRY	No.2
A - B	2x4	DRY	No.2
B - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
G - F	2x4	DRY	No.2
M - J	2x4	DRY	No.2
J - G	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
I - D	2x4	DRY	No.2
SPF			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN
A	TMVW-t	MT20	5.0	6.0
B	TTWW-m	MT20	4.0	6.0
C	TMWW-t	MT20	4.0	4.0
D	TTW-p	MT20	4.0	4.0
E	TMWW-t	MT20	4.0	4.0
F	TMWW+p	MT20	4.0	6.0
G	BMV1+p	MT20	3.0	4.0
H	BMWW-t	MT20	5.0	6.0
I	BMWWW-t	MT20	4.0	9.0
J	BS-t	MT20	3.0	6.0
K	BMWW-t	MT20	4.0	4.0
L	BMWW-t	MT20	5.0	6.0
M	BMV1+p	MT20	3.0	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	MIN. SEAT SIZE: 3-8	MIN. SEAT SIZE: 3-8
M	1632	0	1632	0	0				
G	1632	0	1632	0	0				

UNFACTORED REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
M	1340	744 / 0	305 / 0	0 / 0	0 / 0	291 / 0
G	1340	744 / 0	305 / 0	0 / 0	0 / 0	291 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.62 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-I. *0-1*

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC1)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
M-A	-1574 / 0	0.0	0.0 0.65 (1)	A-L	0 / 2020	0.45 (1)	
A-B	-1499 / 0	-84.3	-84.3 0.34 (1)	L-B	-1180 / 0	0.43 (1)	
B-C	-1678 / 0	-84.3	-84.3 0.46 (1)	B-K	-37 / 0	0.04 (3)	
C-D	-1217 / 0	-84.3	-84.3 0.43 (1)	K-C	0 / 324	0.07 (3)	
D-E	-1215 / 0	-84.3	-84.3 0.41 (1)	C-I	-744 / 0	0.51 (1)	
E-F	-1086 / 0	-84.3	-84.3 0.38 (1)	I-D	0 / 669	0.11 (1)	
G-F	-1572 / 0	0.0	0.0 0.61 (1)	D-E	0 / 108	0.02 (1)	
				H-E	-728 / 0	0.90 (1)	
				H-F	0 / 1347	0.30 (1)	
M-L	0 / 0	-28.0	-28.0 0.21 (3)				
L-K	0 / 1525	-28.0	-28.0 0.39 (2)				
K-J	0 / 1525	-28.0	-28.0 0.38 (2)				
J-I	0 / 1525	-28.0	-28.0 0.38 (2)				
I-H	0 / 997	-28.0	-28.0 0.34 (2)				
H-G	0 / 0	-28.0	-28.0 0.22 (3)				

DESIGN CRITERIA

SPECIFIED LOADS:		
TOP CH.	LL	= 25.6 PSF
	DL	= 3.0 PSF
BOT CH.	LL	= 10.5 PSF
	DL	= 7.0 PSF
TOTAL LOAD		= 46.1 PSF

SPACING = 24.0 IN. CIG

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/999 (0.15")

CSI: TC=0.81 (F-G:1), BC=0.39 (K-L:2), WB=0.90 (E-H:1), SS=0.22 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1997
	822	2284	1655

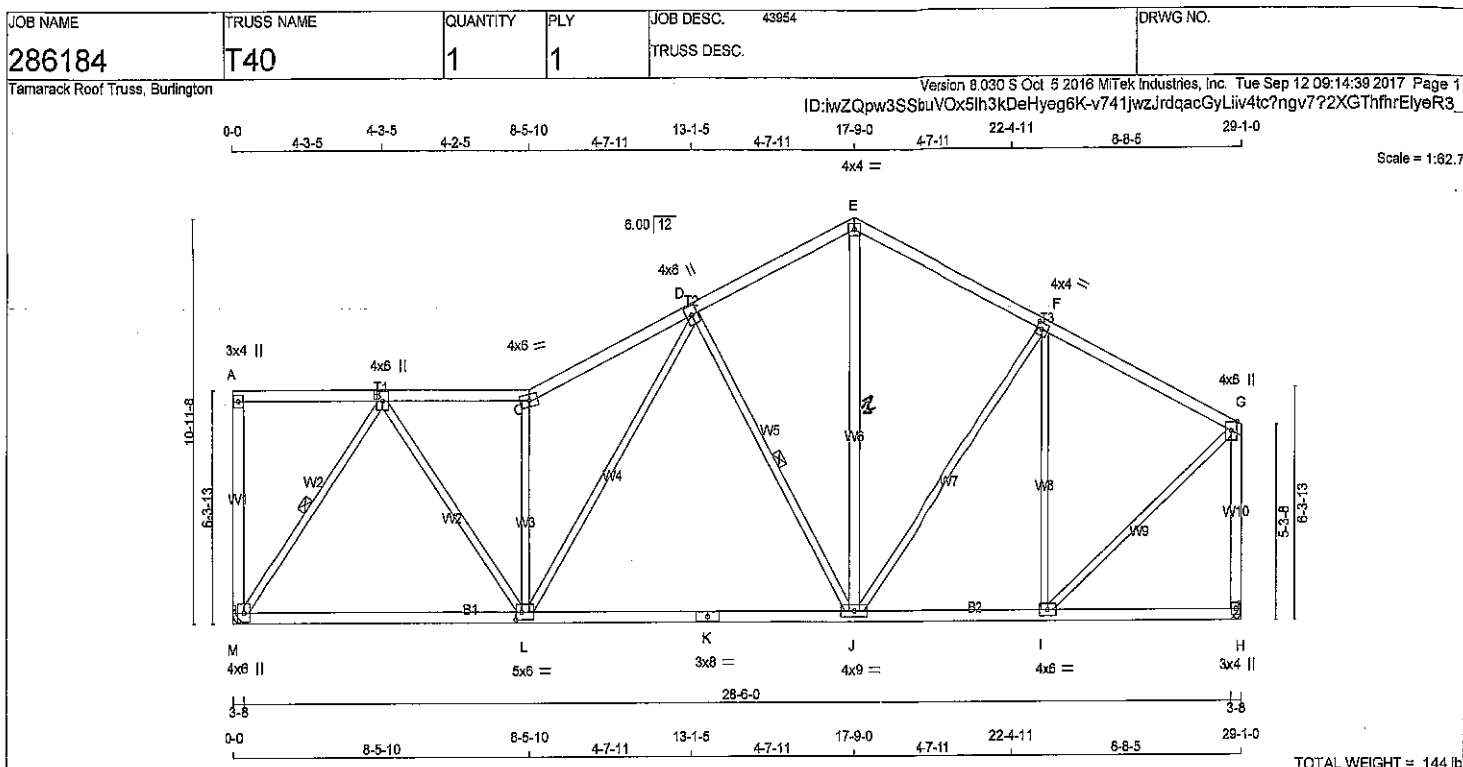
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (L) (INPUT = 0.90)
JSI METAL= 0.46 (J) (INPUT = 1.00)



DWG NO. TAM 46195-17
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS

M - A 2x4 DRY No.2

A - C 2x4 DRY No.2

C - E 2x4 DRY No.2

E - G 2x4 DRY No.2

H - G 2x4 DRY No.2

M - K 2x4 DRY No.2

K - H 2x4 DRY No.2

ALL WEBS 2x3 DRY No.2

EXCEPT

J - E 2x4 DRY No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW+t	MT20	4.0	6.0		
C	TTW-m	MT20	4.0	6.0		
D	TMWW+t	MT20	4.0	6.0		
E	TTW-p	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0	2.00	1.75
G	TMWW+p	MT20	4.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	6.0		
J	BMWW-t	MT20	4.0	6.0		
K	BS-t	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.00
M	BMV1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
M	1632	0	1632	0
H	1632	0	1632	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
M	1340	744 / 0	305 / 0	0 / 0	281 / 0	0 / 0
H	1340	744 / 0	305 / 0	0 / 0	281 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.67 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF B-M, D-J. **E-J**

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
M-A	-136 / 0	0.0	0.0	0.11 (1)	M-B	-1724 / 0	0.62 (1)	7.81
A-B	0 / 0	-84.3	-84.3	0.25 (1)	B-L	0 / 1165	0.26 (1)	10.00
B-C	-1652 / 0	-84.3	-84.3	0.23 (1)	L-C	-1115 / 0	0.71 (1)	4.94
C-D	-1845 / 0	-84.3	-84.3	0.28 (1)	D-J	-718 / 0	0.46 (1)	4.87
D-E	-1222 / 0	-84.3	-84.3	0.26 (1)	J-E	0 / 749	0.12 (1)	5.51
E-F	-1227 / 0	-84.3	-84.3	0.37 (1)	J-F	0 / 115	0.03 (2)	5.35
F-G	-1102 / 0	-84.3	-84.3	0.37 (1)	I-F	-718 / 0	0.93 (1)	5.57
H-G	-1554 / 0	0.0	0.0	0.80 (1)	I-G	0 / 1335	0.30 (1)	8.59
					L-D	0 / 500	0.11 (1)	
M-L	0 / 998	-28.0	-28.0	0.67 (2)				10.00
L-K	0 / 1423	-28.0	-28.0	0.72 (2)				10.00
K-J	0 / 1423	-28.0	-28.0	0.72 (2)				10.00
J-I	0 / 1008	-28.0	-28.0	0.46 (2)				10.00
I-H	0 / 0	-28.0	-28.0	0.19 (2)				10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 3.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014

- CSA 088-09

- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = 1/360 (0.97")

CALCULATED VERT. DEFL.(LL) = 1/999 (0.19")

ALLOWABLE DEFL.(TL) = 1/360 (0.97")

CALCULATED VERT. DEFL.(TL) = 1/999 (0.31")

CSI: TC=0.80 (G-H:1), BC=0.72 (J-L:2), WB=0.93 (F-I:1), SS=0.21 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 616 354 1667 622 2284 1858

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (L) (INPUT = 0.90)

JSI METAL= 0.74 (K) (INPUT = 1.00)



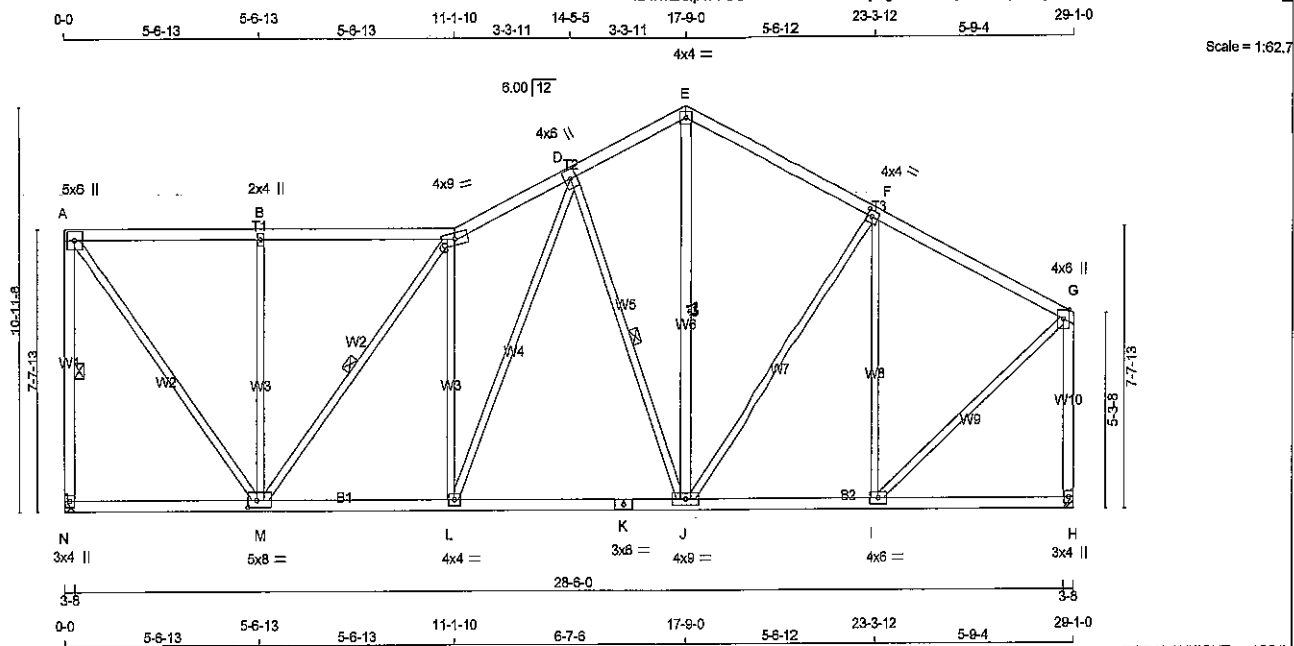
DWG NO. TAM 46196-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286184	T41	1	1	43954	

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 155 lb

LUMBER

N. L. G. A. RULES

CHORDS SIZE

N - A 2x4 DRY No.2

A - C 2x4 DRY No.2

C - E 2x4 DRY No.2

E - G 2x4 DRY No.2

H - G 2x4 DRY No.2

N - K 2x4 DRY No.2

K - H 2x4 DRY No.2

ALL WEBS 2x3 DRY No.2

EXCEPT

J - E 2x4 DRY No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0		
B	TMW+w	MT20	2.0	4.0		
C	TTWW-m	MT20	4.0	9.0		
D	TMWW+t	MT20	4.0	6.0		
E	TTW-p	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0	2.00	1.75
G	TMVW+p	MT20	4.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	6.0		
J	BMWW-t	MT20	4.0	9.0		
K	BS-t	MT20	3.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMWW-t	MT20	5.0	6.0	2.25	3.00
N	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
N	1632	0	1632	0		
H	1632	0	1632	0		

UNFACTORED REACTIONS

JT	COMBINED	MAX./MIN. COMPONENT REACTIONS				
		1ST LOASE	SNOW	LIVE	PERM.LIVE	WIND
N	1340	744 / 0	305 / 0	0 / 0	0 / 0	291 / 0
H	1340	744 / 0	305 / 0	0 / 0	0 / 0	291 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.97 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF A-N, C-M, D-J. *EJS*

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. CS1 (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM	TO			FR-TO			
N-A	-1567 / 0	0.0	0.0	0.41 (1)	5.27	A-M	0 / 1744	0.39 (1)	
A-B	-1057 / 0	-84.3	-84.3	0.35 (1)	5.69	M-B	-580 / 0	0.82 (1)	
B-C	-1057 / 0	-84.3	-84.3	0.35 (1)	5.69	M-C	-800 / 0	0.46 (1)	
C-D	-1709 / 0	-84.3	-84.3	0.15 (1)	4.97	L-C	-411 / 0	0.44 (1)	
D-E	-1210 / 0	-84.3	-84.3	0.13 (1)	5.69	L-D	0 / 819	0.14 (1)	
E-F	-1221 / 0	-84.3	-84.3	0.37 (1)	5.35	D-J	-736 / 0	0.46 (1)	
F-G	-1105 / 0	-84.3	-84.3	0.37 (1)	5.57	J-E	0 / 785	0.13 (1)	
H-G	-1566 / 0	0.0	0.0	0.80 (1)	6.59	J-F	0 / 99	0.02 (1)	
						I-F	-702 / 0	0.92 (1)	
						I-G	0 / 1337	0.30 (1)	
N-M	0 / 0	-28.0	-28.0	0.20 (3)	10.00				
M-L	0 / 1538	-28.0	-28.0	0.38 (2)	10.00				
L-K	0 / 1325	-28.0	-28.0	0.37 (2)	10.00				
K-J	0 / 1325	-28.0	-28.0	0.37 (2)	10.00				
J-I	0 / 1012	-28.0	-28.0	0.34 (2)	10.00				
I-H	0 / 0	-28.0	-28.0	0.21 (3)	10.00				

A circular seal with the word "PROFESSIONAL" in a bold, serif font, arched across the top. The rest of the seal is partially cut off and obscured by the table's border.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.8 PSF
	DL = 3.0 PSF
BOT CH.	LL = 10.5 PSF
	DL = 7.0 PSF
TOTAL LOAD	= 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2012, CBC 2012, ABC 2014
- CSA 088-08
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/999 (0.10")
ALLOWABLE DEFL.(TL) = L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/999 (0.18")

CS1: TC=0.80 (G-H:1), BC=0.38 (L-M:2), WB=0.92 (F-I:1), SS1=0.23 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

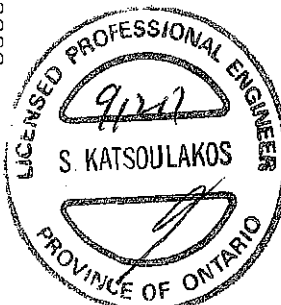
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1867 622 2284 1856

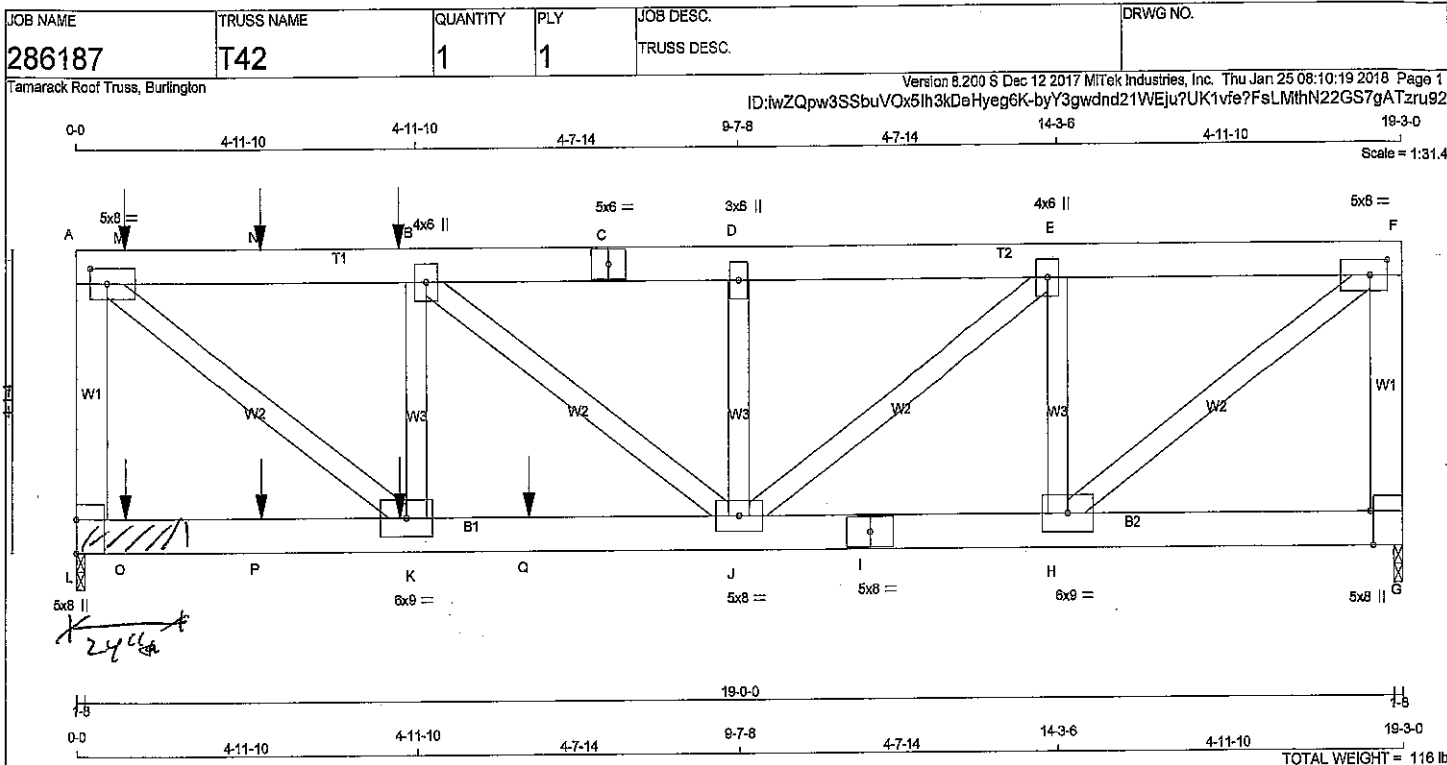
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (G) (INPUT = 0.90)
JSI METAL= 0.54 (C) (INPUT = 1.00)



DWG NO. TAM 46197-17
STRUCTURAL
COMPONENT ONLY



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
L - A	2x6	DRY	No.2
A - C	2x6	DRY	2100F 1.8E
C - F	2x6	DRY	2100F 1.8E
G - I	2x6	DRY	No.2
I - J	2x6	DRY	2100F 1.8E
J - G	2x6	DRY	2100F 1.8E

ALL WEBS 2x4 DRY No.2 SPF
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWV-t	MT20	5.0	8.0	2.50	3.00
B	TMWV-t	MT20	4.0	6.0		
C	TS-t	MT20	5.0	6.0		
D	TMWV-t	MT20	3.0	6.0		
E	TMWV-t	MT20	4.0	6.0		
F	TMWV-t	MT20	5.0	6.0	2.50	3.00
G	BMV1-p	MT20	5.0	6.0	Edge	0.50
H	BMWV-t	MT20	6.0	9.0		
I	BS-t	MT20	5.0	6.0		
J	BMWV-t	MT20	5.0	6.0		
K	BMWV-t	MT20	6.0	9.0		
L	BMV1-p	MT20	5.0	6.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 112.8 lbs FACTORED DOWN AT 8-12, AND 101.4 lbs FACTORED DOWN AT 2-8-12, AND 101.4 lbs FACTORED DOWN AT 4-8-12 ON TOP CHORD, AND 78.5 lbs FACTORED DOWN AT 8-12, 69.9 lbs FACTORED DOWN AT 2-8-12, AND 68.9 lbs FACTORED DOWN AT 4-8-12, AND 1278.2 lbs FACTORED DOWN AT 6-7-0 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
L	2285	0	2285	0	1-8	1-8
G	1580	0	1580	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
L	1875	1080 / 0	414 / 0	0 / 0	0 / 0	401 / 0	0 / 0
G	1289	729 / 0	284 / 0	0 / 0	0 / 0	275 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.83 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 MAX (PLF)	MAX. UNBRACED LENGTH FR-TO	MAX. FACTORED FORCE (LBS)
FR-TO				
L-A	-2265 / 0	0.0	0.0	6.77
A-M	-2859 / 0	-84.3	-84.3	5.91
M-N	-2659 / 0	-84.3	-84.3	5.91
N-B	-2859 / 0	-84.3	-84.3	5.91
B-C	-2850 / 0	-84.3	-84.3	5.83
C-D	-2850 / 0	-84.3	-84.3	5.83
D-E	-2850 / 0	-84.3	-84.3	5.83
E-F	-1810 / 0	-84.3	-84.3	6.25
G-F	-1502 / 0	0.0	0.0	7.81
L-O	0 / 0	-28.0	-28.0	10.00
O-P	0 / 0	-28.0	-28.0	10.00
P-K	0 / 0	-28.0	-28.0	10.00
K-Q	0 / 2659	-28.0	-28.0	10.00
Q-J	0 / 2659	-28.0	-28.0	10.00
J-I	0 / 1810	-28.0	-28.0	10.00
I-H	0 / 1810	-28.0	-28.0	10.00
H-G	0 / 0	-28.0	-28.0	10.00

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TOTAL
B	4-8-12	-101	-101	-	BACK	VERT	12578
K	4-8-12	-40	-70	-	BACK	VERT	
M	8-12	-113	-113	-	BACK	VERT	
N	2-8-12	-101	-101	-	BACK	VERT	
O	8-12	-44	-77	-	BACK	VERT	
P	2-8-12	-40	-70	-	BACK	VERT	
Q	6-7-0	-1278	-1278	-	BACK	VERT	

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.64")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.10")
ALLOWABLE DEFL.(TL) = $L/360$ (0.64")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.16")

CSF TC=0.33/1.00 (A-L:1), BC=0.54/1.00 (J-K:1), WB=0.58/1.00 (A-K:1), SS=0.81/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

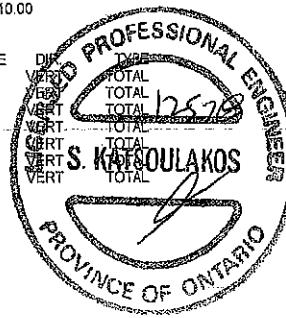
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (A) (INPUT = 0.90)
JSI METAL= 0.83 (I) (INPUT = 1.00)



DRWG NO. TAM 5979-18
STRUCTURAL
COMPONENT ONLY

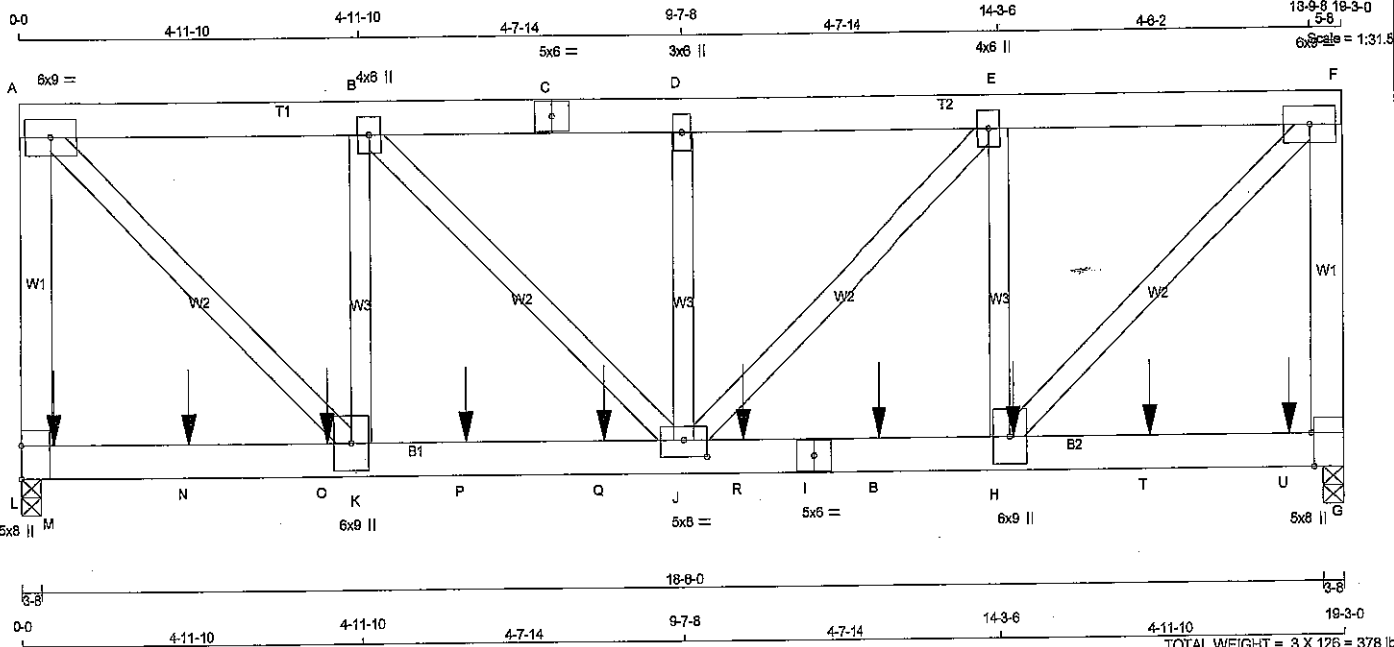
SCAB JOINT # L-1 WITH 2 X 6 #2
SPF 24" LONG L SIDE(S) USING 2
ROW(S) 3/16" COMMON WIRE NAILS
AT 3" O/C STAGGERED (15 NAILS/SCAB)
REQUIRED FOR 7/8" BRG PROMSION.

JOB NAME 286187	TRUSS NAME T43	QUANTITY 1	PLY 3	JOB DESC. TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
L - A	2x6	DRY No.2	SPF	
A - C	2x6	DRY 2100F 1.8E	SPF	
C - F	2x6	DRY 2100F 1.8E	SPF	
G - F	2x6	DRY No.2	SPF	
L - I	2x6	DRY 2100F 1.8E	SPF	
I - G	2x6	DRY 2100F 1.8E	SPF	

ALL WEBS 2x4 DRY No.2 SPF
DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
L - A	2 12	SIDE(33.1)
A - C	2 12	TOP
C - F	2 12	TOP
F - G	2 12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
L - I	2 8	SIDE(418.5)
I - G	2 8	SIDE(535.9)
WEBS : (0.122"x3") SPIRAL NAILS		
2x4	1 6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMW+t	MT20	6.0	9.0		
B	TMW+t	MT20	4.0	6.0		
C	TS-t	MT20	5.0	5.0		
D	TMW+w	MT20	3.0	8.0		
E	TMW+t	MT20	4.0	6.0		
F	TMV+t	MT20	6.0	9.0		
G	BMV1+p	MT20	5.0	8.0	Edge	0.50
H	BMW+t	MT20	6.0	9.0		
I	BS-t	MT20	5.0	6.0		
J	BMW+t	MT20	5.0	8.0	2.75	4.00
K	BMW+t	MT20	6.0	9.0		
L	BMV1+p	MT20	5.0	8.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQ'D BRG
JT	VERT	HORZ	DOWN	HORZ
L	9248	0	9248	0
G	8967	0	8967	0

UNFACTORED REACTIONS

	1ST CASE	MAX/MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
L	7535	4277 / 0	1854 / 0
G	7305	4149 / 0	1602 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.70 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	MAX. FACTORED CSI (LC)
FR-TO		FROM TO		FR-TO			
L-A	-7147 / 0	0.0	0.0 0.58 (1)	H-F	0 / 10170	6.87	0.54 (1)
A-B	-7415 / 0	-84.3	-84.3 0.04 (1)	A-K	0 / 10152	6.22	0.54 (1)
B-C	-9390 / 0	-84.3	-84.3 0.05 (1)	H-E	-2353 / 0	5.70	0.19 (1)
C-D	-9390 / 0	-84.3	-84.3 0.05 (1)	K-B	-2887 / 0	5.70	0.19 (1)
D-E	-9390 / 0	-84.3	-84.3 0.05 (1)	J-E	0 / 2770	5.70	0.15 (1)
E-F	-7429 / 0	-84.3	-84.3 0.04 (1)	B-J	0 / 2790	8.22	0.15 (1)
G-F	-7180 / 0	0.0	0.0 0.58 (1)	J-D	-401 / 0	6.67	0.03 (1)
L-M	0 / 0	-28.0	-28.0 0.23 (1)				
M-N	0 / 0	-28.0	-28.0 0.23 (1)				
N-O	0 / 0	-28.0	-28.0 0.23 (1)				
O-K	0 / 0	-28.0	-28.0 0.23 (1)				
K-P	0 / 7415	-28.0	-28.0 0.35 (1)				
P-Q	0 / 7415	-28.0	-28.0 0.35 (1)				
Q-J	0 / 7415	-28.0	-28.0 0.35 (1)				
J-R	0 / 7429	-28.0	-28.0 0.32 (1)				
R-I	0 / 7429	-28.0	-28.0 0.32 (1)				
I-S	0 / 7429	-28.0	-28.0 0.32 (1)				
S-H	0 / 7429	-28.0	-28.0 0.32 (1)				
H-T	0 / 0	-28.0	-28.0 0.20 (1)				
T-U	0 / 0	-28.0	-28.0 0.20 (1)				
U-G	0 / 0	-28.0	-28.0 0.20 (1)				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE
H	14-5-12	-1604	-1604	FRONT
M	5-12	-1612	-1612	FRONT
N	2-5-12	-1604	-1604	FRONT
O	4-5-12	-1604	-1604	FRONT
P	6-5-12	-1604	-1604	FRONT
Q	8-5-12	-1604	-1604	FRONT
R	10-5-12	-1604	-1604	FRONT
S	12-5-12	-1604	-1604	FRONT
T	16-5-12	-1604	-1604	FRONT
U	18-5-12	-1608	-1608	FRONT

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
	DL = 3.0 PSF
BOT CH.	LL = 10.5 PSF
	DL = 7.0 PSF
TOTAL LOAD	= 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 085-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.64")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.08")
ALLOWABLE DEFL.(TL) = $L/360$ (0.64")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.12")

CSI: TC=0.58/1.00 (F-G:1), BC=0.35/1.00 (J-K:1),
WB=0.54/1.00 (F-H:1), SS=0.57/1.00 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1857
	822	2284	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.56 (I) (INPUT = 1.00)

DRWG NO. TAM 5094
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

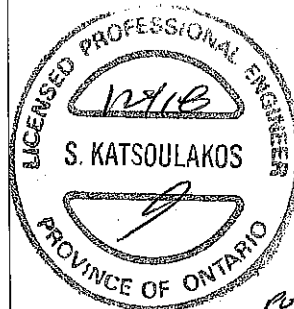
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286187	T43	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

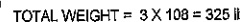
Version 8.200 S Dec 12 2017 MITek Industries, Inc. Tue Jan 23 08:39:28 2018 Page 2
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HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 1611.6 lbs FACTORED DOWN AT 5-12,
1604.3 lbs FACTORED DOWN AT 2-5-12, 1604.3
lbs FACTORED DOWN AT 4-5-12, 1604.3 lbs
FACTORED DOWN AT 6-5-12, 1604.3 lbs
FACTORED DOWN AT 8-5-12, 1604.3 lbs
FACTORED DOWN AT 10-5-12, 1604.3 lbs
FACTORED DOWN AT 12-5-12, 1604.3 lbs
FACTORED DOWN AT 14-5-12, AND 1604.3 lbs
FACTORED DOWN AT 16-5-12, AND 1607.6 lbs
FACTORED DOWN AT 18-5-12 ON BOTTOM
CHORD. DESIGN FOR UNSPECIFIED
CONNECTION(S) IS DELEGATED TO THE
BUILDING DESIGNER.



DWG NO. TAM 5094 -18
STRUCTURAL
COMPONENT ONLY

 $\frac{13}{13}$

TOTAL LOAD CASES: (4)

M-R	0/0	-28.0	-28.0	0.25 (1)	10.00
R-L	0/0	-28.0	-28.0	0.25 (1)	10.00
L-S	0/6831	-28.0	-28.0	0.36 (1)	10.00
S-T	0/6831	-28.0	-28.0	0.36 (1)	10.00
T-K	0/6831	-28.0	-28.0	0.36 (1)	10.00
K-U	0/25189	-28.0	-28.0	0.66 (1)	10.00
U-V	0/25189	-28.0	-28.0	0.66 (1)	10.00
V-W	0/25189	-28.0	-28.0	0.66 (1)	10.00
W-J	0/25189	-28.0	-28.0	0.66 (1)	10.00
J-X	0/24904	-28.0	-28.0	0.58 (1)	10.00
X-I	0/24804	-28.0	-28.0	0.58 (1)	10.00
I-Y	0/24904	-28.0	-28.0	0.58 (1)	10.00
Y-H	0/24904	-28.0	-28.0	0.58 (1)	10.00
H-Z	0/7301	-28.0	-28.0	0.30 (1)	10.00
Z-AA	0/7301	-28.0	-28.0	0.30 (1)	10.00
AA-G	0/7301	-28.0	-28.0	0.30 (1)	10.00

A circular professional engineer seal for S. Katsoulakos. The outer ring contains the text "REGISTERED PROFESSIONAL ENGINEER" at the top and "PROVINCE OF ONTARIO" at the bottom. The center features a stylized "S" logo with the number "118" written across it, and the name "S. KATSOULAKOS" printed below.

DWG NO. TAM 5095-18
STRUCTURAL
COMPONENT ONLY

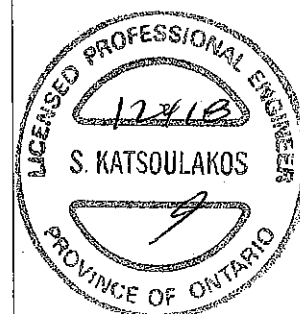
JOB NAME 286187	TRUSS NAME T44	QUANTITY 1	PLY 3	JOB DESC. TRUSS DESC.	DRWG NO.
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HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1.8 lbs FACTORED DOWN AT 1-1-8, 1.0 lbs FACTORED DOWN AT 3-0-12, 1.0 lbs FACTORED DOWN AT 5-0-12, AND 1.0 lbs FACTORED DOWN AT 7-0-12, AND 1.0 lbs FACTORED DOWN AT 7-9-12 ON TOP CHORD, AND 1606.4 lbs FACTORED DOWN AT 10-4, 1.0 lbs FACTORED DOWN AT 1-0-12, 1604.3 lbs FACTORED DOWN AT 2-10-4, 1.0 lbs FACTORED DOWN AT 3-0-12, 1604.3 lbs FACTORED DOWN AT 4-10-4, 1.0 lbs FACTORED DOWN AT 5-0-12, 1604.3 lbs FACTORED DOWN AT 8-10-4, 1.0 lbs FACTORED DOWN AT 7-0-12, 1.0 lbs FACTORED DOWN AT 7-9-12, 1604.3 lbs FACTORED DOWN AT 8-10-4, 1604.3 lbs FACTORED DOWN AT 10-10-4, 1604.3 lbs FACTORED DOWN AT 12-10-4, 1604.3 lbs FACTORED DOWN AT 14-10-4, AND 1604.3 lbs FACTORED DOWN AT 16-10-4, AND 1607.6 lbs FACTORED DOWN AT 18-10-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)								
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	
B	1-1-8	-2	-2	---	FRONT	VERT	TOTAL	
H	14-10-4	-1604	-1604	---	BACK	VERT	TOTAL	
N	3-0-12	1	1	---	FRONT	VERT	TOTAL	
O	5-0-12	1	1	---	FRONT	VERT	TOTAL	
P	7-0-12	1	1	---	FRONT	VERT	TOTAL	
Q	7-9-12	1	1	---	FRONT	VERT	TOTAL	
R	10-4	-1608	-1608	---	BACK	VERT	TOTAL	
R	1-0-12	1	1	---	FRONT	VERT	TOTAL	
S	2-10-4	-1604	-1604	---	BACK	VERT	TOTAL	
S	3-0-12	1	1	---	FRONT	VERT	TOTAL	
T	4-10-4	-1604	-1604	---	BACK	VERT	TOTAL	
T	5-0-12	1	1	---	FRONT	VERT	TOTAL	
U	6-10-4	-1604	-1604	---	BACK	VERT	TOTAL	
U	7-0-12	1	1	---	FRONT	VERT	TOTAL	
V	7-9-12	1	1	---	FRONT	VERT	TOTAL	
W	8-10-4	-1604	-1604	---	BACK	VERT	TOTAL	
X	10-10-4	-1604	-1604	---	BACK	VERT	TOTAL	
Y	12-10-4	-1604	-1604	---	BACK	VERT	TOTAL	
Z	16-10-4	-1604	-1604	---	BACK	VERT	TOTAL	
AA	16-10-4	-1608	-1608	---	BACK	VERT	TOTAL	



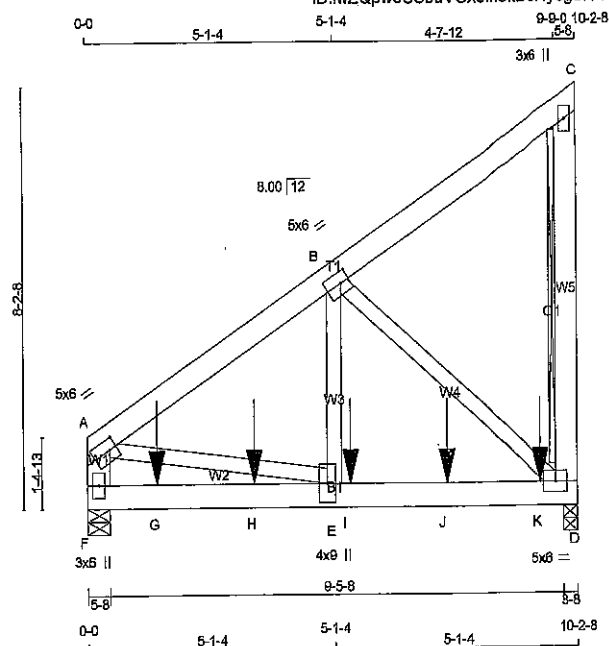
DWG NO. TAM 5095-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286186	T360A	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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Scale = 1:45.0

TOTAL WEIGHT = 3 X 71 = 214 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x6 DRY	No.2	SPF		
D - C	2x6 DRY	No.2	SPF		
F - A	2x6 DRY	No.2	SPF		
F - D	2x6 DRY	No.2	SPF		
ALL WEBS	2x4 DRY	No.2	SPF		
EXCEPT					

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	2 12	TOP
C - D	2 12	TOP
F - A	2 12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F - D	2 7	SIDE (549.3)
WEBS : (0.122"x3") SPIRAL NAILS		
2x4	1 8	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	8.0	2.50	1.75
B	TMVW-t	MT20	5.0	8.0	2.50	2.25
C	TMV+p	MT20	3.0	6.0		
D	BMVW-t	MT20	5.0	8.0		
E	BMVW-t	MT20	4.0	9.0		
F	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1849.6 lbs FACTORED DOWN AT 1-5-4, 1735.3 lbs FACTORED DOWN AT 3-5-4, 1735.3 lbs FACTORED DOWN AT 5-5-4, AND 1735.3 lbs FACTORED DOWN AT 7-5-4, AND 1738.5 lbs FACTORED DOWN AT 9-5-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
D	5214	0	5214	0
F	4726	0	4726	0

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED	MAX./MIN. SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	4248	2412 / 0	932 / 0	0 / 0	0 / 0	904 / 0	0 / 0
F	3838	2199 / 0	829 / 0	0 / 0	0 / 0	811 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x6 DRY SPF No.2 T-BRACE AT C-D

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

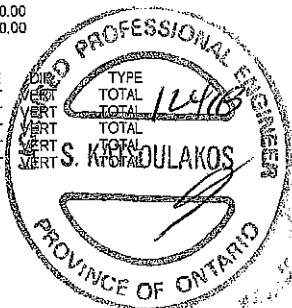
LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	-4414 / 0	-84.3 -84.3	0.06 (1)	6.25	E-B	0 / 5074	0.27 (1)
B-C	-16 / 4	-84.3 -84.3	0.05 (1)	6.25	B-D	-4819 / 0	0.80 (1)
D-C	-186 / 0	0.0 0.0	0.01 (1)	7.81	A-E	0 / 3742	0.20 (1)
F-A	-3291 / 0	0.0 0.0	0.07 (1)	7.81			
F-G	0 / 0	-28.0 -28.0	0.60 (1)	10.00			
G-H	0 / 0	-28.0 -28.0	0.60 (1)	10.00			
H-E	0 / 0	-28.0 -28.0	0.60 (1)	10.00			
E-I	0 / 3688	-28.0 -28.0	0.75 (1)	10.00			
I-J	0 / 3688	-28.0 -28.0	0.75 (1)	10.00			
J-K	0 / 3688	-28.0 -28.0	0.75 (1)	10.00			
K-D	0 / 3688	-28.0 -28.0	0.75 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE
G	1-5-4	-1850	-1850		FRONT
H	3-5-4	-1735	-1735		FRONT
I	5-5-4	-1735	-1735		FRONT
J	7-5-4	-1735	-1735		FRONT
K	9-5-4	-1739	-1739		FRONT



DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPC 2011

(55% OF 31.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.07/1.00 (A-F:1), BC=0.75/1.00 (D-E:1), WS=0.80/1.00 (B-D:1), SS=0.69/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1687 822	2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.83 (A) (INPUT = 0.90)
JSI METAL= 0.40 (E) (INPUT = 1.00)

DRWG NO. TAM 5098-18
STRUCTURAL
COMPONENT ONLY

LUL/LUS/LJS/HUS/HHUS/HGUS



Standard and Double-Shear Joist Hangers



This product is preferable to similar connectors because of
a) easier installation, b) higher capacities, c) lower installed
cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 258–259.

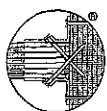
Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp.20–24.

Installation:

- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1 1/2" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on p. 126.



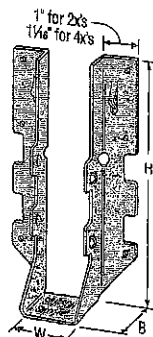
Double-Shear Nailing Top View



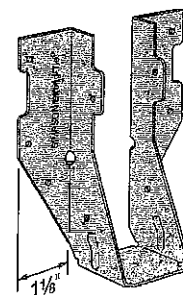
Double-Shear Nailing Side View; Do not bend tab



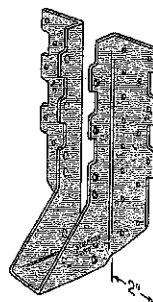
Dome Double-Shear Nailing Side View (available on some models) U.S. Patent 5,603,580



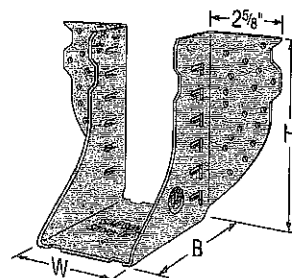
LUS28



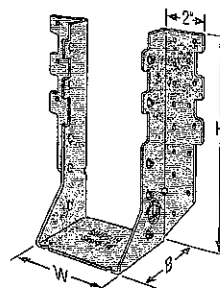
LU26L



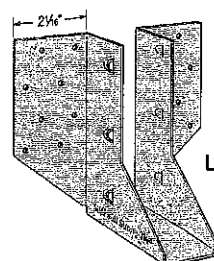
HUS210 (HUS26, HUS28, and HHUS similar)



HGUS28-2

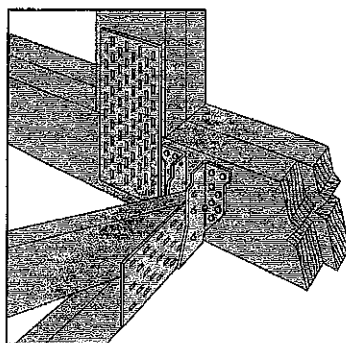


HHUS210-2



LJS26DS

Typical HUS26 Installation with Reduced Heel Height (Truss Designer to provide fastener quantity for connecting multiple members together)



Plated Truss Connectors

LUL/LUS/LJS/HUS/HHUS/HGUS

HHUS/HGUS

See Hanger Options Information on pp. 125–127.

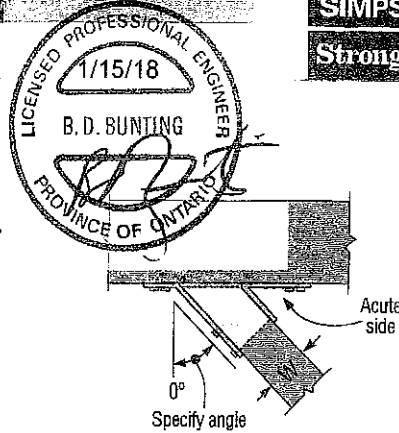
HHUS — Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.62 of the table value
- The joist must be bevel-cut to allow for double-shear nailing

HGUS — Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Joist	Down Resistance	Uplift
W < 2"	Bevel or square cut	0.62 of table value	0.46 of table value
2" < W < 6"	Bevel cut	0.67 of table value	0.41 of table value
2" < W < 6"	Square cut	0.46 of table value	0.41 of table value
W > 6"	Bevel cut	0.75 of table value	0.41 of table value



**Top View HHUS Hanger
Skewed Right**
(joist must be bevel cut)
All joist nails installed on the
outside angle (non-acute side).

Standard and Double-Shear Joist Hangers (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance			
		W	H	B	d _e ³	Header	Joist	D-Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
								(K _D = 1.15) lb.	(K _D = 1.00) lb.	(K _D = 1.15) lb.	(K _D = 1.00) lb.
Single 2x Sizes											
LUS24	18	1 1/8	3 1/8	1 1/4	2 1/4	(4) 10d	(2) 10d	710	1625	645	1155
								3.16	7.23	2.87	5.14
LU24L	22	1 1/8	3	1 1/8	2 1/8	(4) 10d	(2) 10d x 1 1/2"	360	1020	320	725
								1.60	4.54	1.42	3.22
LU26L	22	1 1/8	5	1 1/8	4 1/8	(6) 10d	(4) 10d x 1 1/2"	720	1605	645	1140
								3.20	7.14	2.87	5.07
LUS26	18	1 1/8	4 3/4	1 3/4	3 3/4	(4) 10d	(4) 10d	1420	2170	1290	1630
								6.32	9.65	5.74	7.25
HUS26	16	1 1/8	5 1/8	3	3 1/8	(14) 16d	(6) 16d	2705	4940	2065	3875
								11.30	21.97	9.20	17.24
LJS26DS	18	1 1/8	5	3 1/2	4 1/8	(16) 16d	(6) 16d	2055	4265	1460	4115
								9.14	18.97	6.49	18.31
HGUS26	12	1 1/8	5 1/8	5	4 1/8	(20) 16d	(8) 16d	2685	6625	2685	5700
								11.96	29.51	11.96	25.35
LU28L	20	1 1/8	6 3/8	1 1/8	5 1/8	(8) 10d	(6) 10d x 1 1/2"	1140	2185	1020	1550
								5.07	9.72	4.54	6.89
LUS28	18	1 1/8	6 1/8	1 1/4	3 3/4	(6) 10d	(4) 10d	1420	2520	1290	1790
								6.32	11.21	5.74	7.96
HUS28	16	1 1/8	7 1/8	3	6 1/8	(22) 16d	(8) 16d	3605	5365	2675	4345
								16.04	23.86	11.90	19.33
HGUS28	12	1 1/8	7 1/8	5	6 1/8	(36) 16d	(12) 16d	3310	7675	3310	6900
								14.74	34.19	14.74	30.73
LU210L	20	1 1/8	8	1 1/8	7 1/8	(10) 10d	(6) 10d x 1 1/2"	1140	2495	1020	1770
								5.07	11.10	4.54	7.87
LUS210	18	1 1/8	7 1/8	1 1/4	3 3/4	(8) 10d	(4) 10d	1420	2785	1290	2210
								6.32	12.39	5.74	9.83

1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.
3. d_e is the distance from the bearing seat to the top joist nail.
4. Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-C-N10TRSSCN and/or see installation notes.
5. Nails: 16d = 0.162" dia. x 3 1/4" long. See pp. 27–28 for other nail sizes and information.

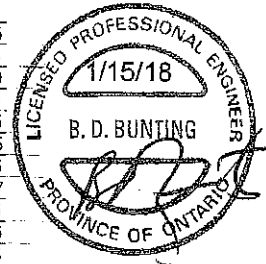
Face-Mount Hangers

SIMPSON
Strong-Tie

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

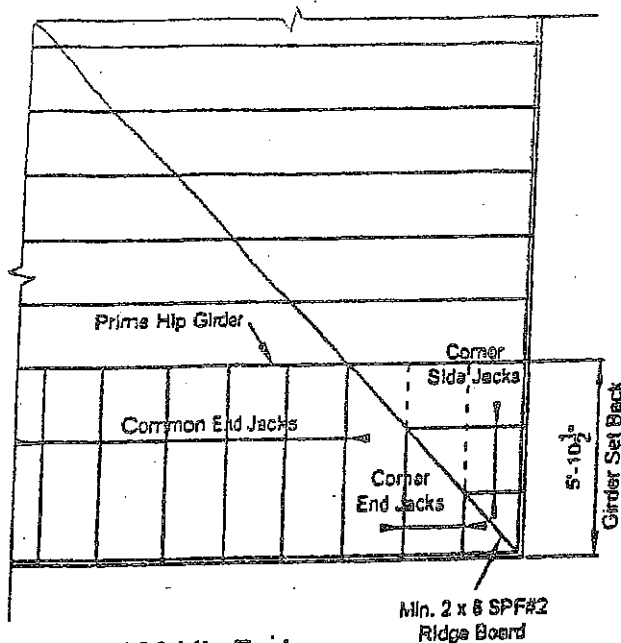
Model No.	Ga.	Dimensions (In.)				Fasteners		Factored Resistance			
		W	H	B	d _g ³	Header	Joist	D-Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
								(K _D = 1.15) lb. kN	(K _D = 1.00) lb. kN	(K _D = 1.15) lb. kN	(K _D = 1.00) lb. kN
Double 2x Sizes											
LUS24-2	18	3 1/8	3 1/8	2	1 1/2	(4) 16d	(2) 16d	835 3.71	2020 8.99	590 2.62	1435 6.38
LUS26-2	18	3 1/8	4 1/8	2	4	(4) 16d	(4) 16d	1720 7.65	2595 11.54	1545 6.87	1920 8.54
HHUS26-2	14	3 1/8	5 1/8	3	3 1/8	(14) 16d	(6) 16d	2850 12.68	7335 32.63	2065 9.20	5205 23.15
HGUS26-2	12	3 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385 19.51	8950 39.81	3110 13.83	6355 28.27
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720 7.65	3325 14.79	1545 6.87	2575 11.45
HHUS28-2	14	3 1/8	7 1/8	3	6 1/8	(22) 16d	(8) 16d	3765 16.75	8940 39.77	2675 11.90	6345 28.22
HGUS28-2	12	3 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070 27.00	12980 57.74	4310 19.17	9215 40.99
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580 11.48	4500 20.02	2320 10.32	3195 14.21
HHUS210-2	14	3 1/8	9 1/8	3	8	(30) 16d	(10) 16d	4670 20.77	9660 42.97	4235 18.84	7000 31.14
HGUS210-2	12	3 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	6840 30.43	14015 62.34	4855 21.60	10270 45.69
Triple 2x Sizes											
HGUS26-3	12	4 1/8	5 1/2	4	4 1/8	(20) 16d	(8) 16d	4385 19.51	8950 39.81	3110 13.83	6355 28.27
HGUS28-3	12	4 1/8	7 1/4	4	6 1/8	(36) 16d	(12) 16d	6070 27.00	12980 57.74	4310 19.17	9215 40.99
HHUS210-3	14	4 1/8	9	3	7 1/8	(30) 16d	(10) 16d	4670 20.77	9670 43.02	4235 18.84	6865 30.54
HGUS210-3	12	4 1/8	9 1/4	4	8 1/8	(46) 16d	(16) 16d	6840 30.43	14645 65.14	4855 21.60	10400 46.26
Quadruple 2x Sizes											
HGUS26-4	12	6 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385 19.51	8950 39.81	3110 13.83	6355 28.27
HGUS28-4	12	6 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070 27.00	12980 57.74	4310 19.17	9215 40.99
HHUS210-4	14	6 1/8	8 1/8	3	7 1/8	(30) 16d	(10) 16d	4670 20.77	10155 45.17	4235 18.84	7210 32.07
HGUS210-4	12	6 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	6840 30.43	14645 65.14	4855 21.60	10400 46.26
HGUS212-4	12	6 1/8	10 1/8	4	10 1/8	(56) 16d	(20) 16d	7640 33.98	14995 66.70	5425 24.13	10645 47.35
HGUS214-4	12	6 1/8	12 1/8	4	11 1/8	(66) 16d	(22) 16d	10130 45.06	16400 72.95	7195 32.00	11645 51.80
4x Sizes											
LUS46	18	3 1/8	4 1/8	2	3 1/8	(4) 16d	(4) 16d	1720 7.65	2595 11.54	1545 6.87	1920 8.54
HHUS46	14	3 1/8	5 1/8	3	3 1/8	(14) 16d	(6) 16d	2540 11.30	7335 32.63	2065 9.20	5205 23.15
HGUS46	12	3 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385 19.51	8950 39.81	3110 13.83	6355 28.27
LUS48	18	3 1/8	6 1/8	2	3 1/8	(6) 16d	(4) 16d	1720 7.65	3325 14.79	1545 6.87	2575 11.45
HHUS48	14	3 1/8	7 1/8	3	6 1/8	(22) 16d	(8) 16d	3765 16.75	8940 39.77	2675 11.90	6345 28.22
HGUS48	12	3 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070 27.00	12980 57.74	4310 19.17	9215 40.99
LUS410	18	3 1/8	8 1/8	2	5 1/8	(8) 16d	(6) 16d	2580 11.48	4500 20.02	2320 10.32	3195 14.21
HGUS410	12	3 1/8	9	4	8 1/8	(46) 16d	(16) 16d	6840 30.43	14015 62.34	4855 21.60	10270 45.69
HGUS412	12	3 1/8	10 1/8	4	10 1/8	(56) 16d	(20) 16d	7640 33.98	14995 66.70	5425 24.13	10645 47.35
HGUS414	12	3 1/8	12 1/8	4	11 1/8	(66) 16d	(22) 16d	10130 45.06	16400 72.95	7195 32.00	11645 51.80



Plated Truss Connectors

MICRO CITY ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242



45° Hip End

LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2

BOTTOM CHORD : 2 x 4 SPF#2

WEBS : 2 x 3 SPF#2

UNLESS OTHERWISE SHOWN

DESIGN LOAD:

TOP CHORD LIVE LOAD : 34.8 P.S.F.

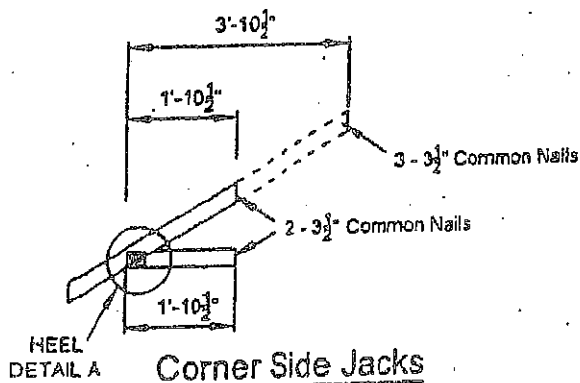
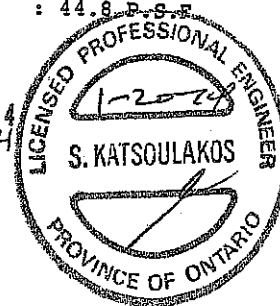
TOP CHORD DEAD LOAD : 3.0 P.S.F.

BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.

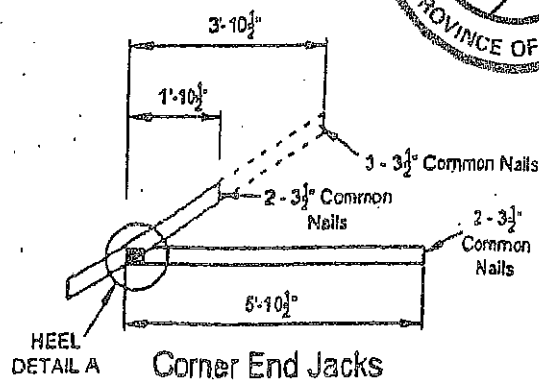
BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD : 44.8 P.S.F.

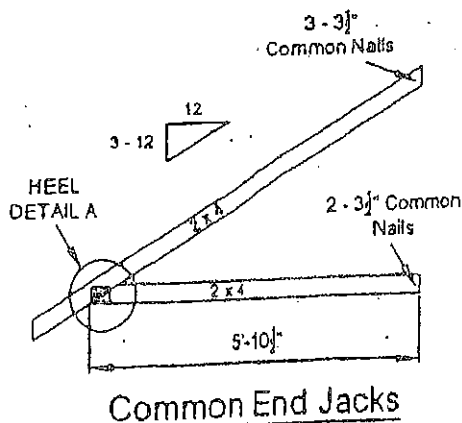
DWG NO TAM 3495.14
STRUCTURAL
COMPONENT ONLY



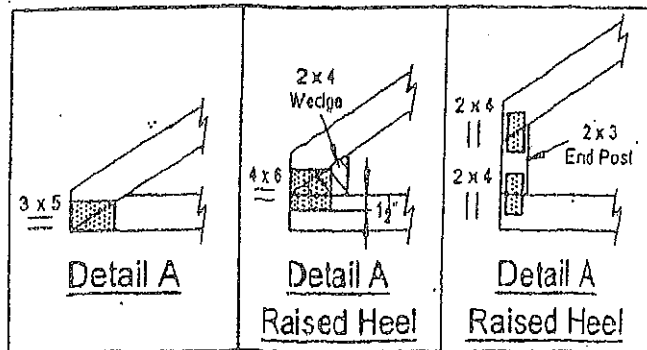
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

MICRO CITY ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

LUMBER SPECIFICATION

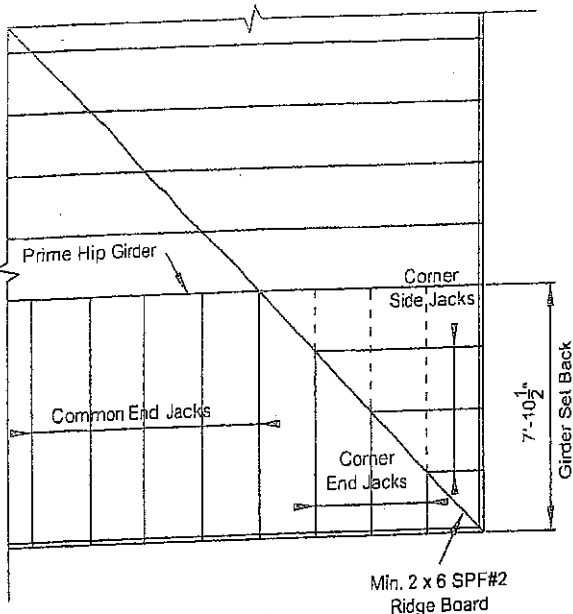
TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

DESIGN LOAD:

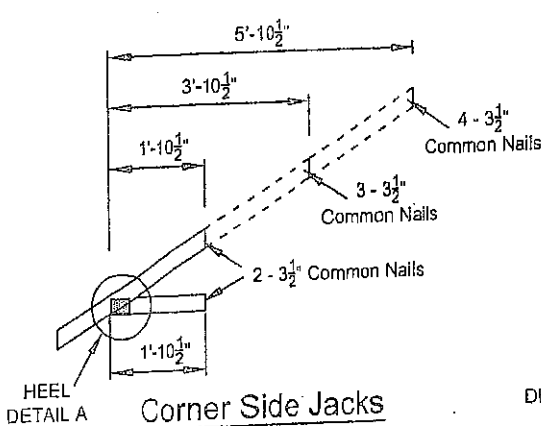
TOP CHORD LIVE LOAD : 40.5 P.S.F.
TOP CHORD DEAD LOAD : 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD : 50.5 P.S.F.

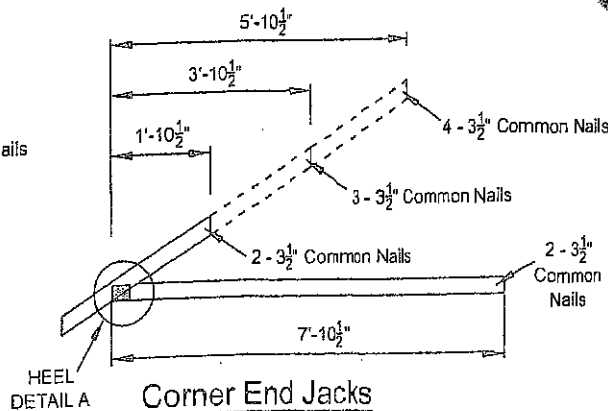
DWG NO TAM 3504.1
STRUCTURAL
COMPONENT ONLY



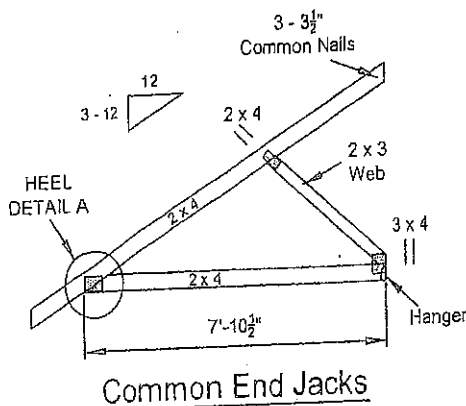
45° Hip End



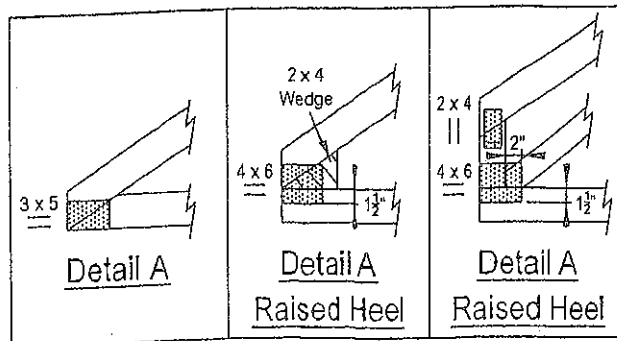
Corner Side Jacks



Corner End Jacks



Common End Jacks



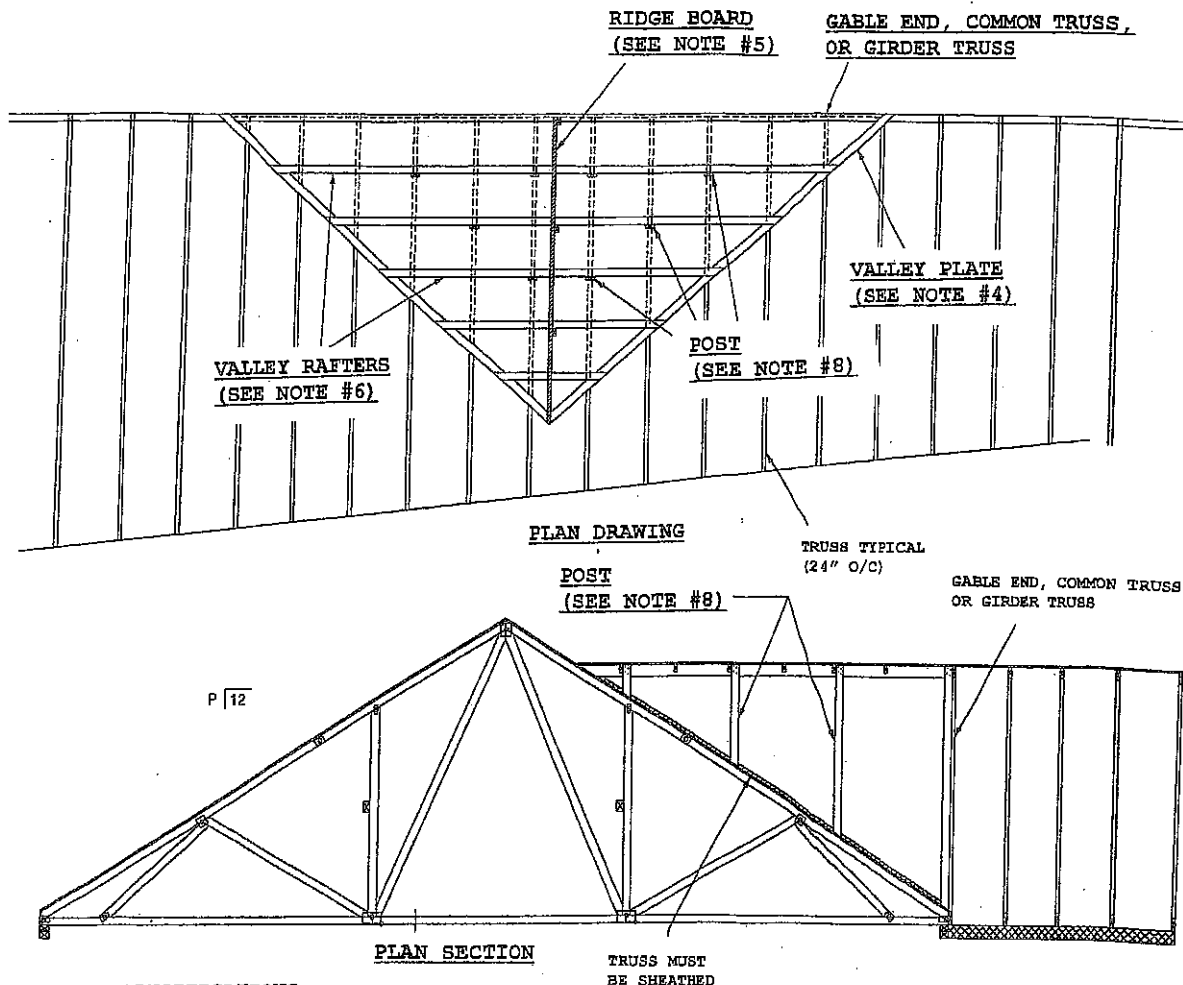
NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

MICRO CITY ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

CONVENTIONAL VALLEY FRAMING DETAIL



GENERAL SPECIFICATIONS:

- (1) WITH THE BASE TRUSSES ERECTED (INSTALLED), APPLY SHEATHING TOP CHORD OF SUPPORTING (BASE) TRUSSES.
- (2) BRACE BOTTOM CHORD AND WEB MEMBERS AS PER PRE-ENGINEERED TRUSS DESIGNS.
- (3) DEFINE VALLEY RIDGE BY RUNNING A LEVEL STRING FROM THE INTERSECTING RIDGE OF THE (a) GABLE END, (b) GIRDER TRUSS OR (c) COMMON TRUSS TO THE ROOF SHEATHING.
- (4) INSTALL 2 X 6 VALLEY PLATES ON FLAT. FASTEN TO EACH SUPPORTING TRUSS WITH (2) 16d (3.5" X 0.131") NAILS.
- (5) SET A 2 X 6 #2 RIDGE BOARD (MAX. 10'-0" RIDGE) OR 2 X 8 #2 SPF RIDGE BOARD (MAX. 20'-0" RIDGE). SUPPORT RIDGE BOARD WITH 2 X 4 POSTS SPACED 48" O/C. BEVEL BOTTOM OF POST TO SET EVENLY ON THE SHEATHING. FASTEN POST TO RIDGE WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST TO ROOF SHEATHING WITH (3) 10d (3" X 0.131") TOE-NAILS.
- (6) FRAME VALLEY RAFTERS FROM VALLEY PLATE TO RIDGE BOARD. MAXIMUM RAFTER SPACING IS 24" O/C. FASTEN VALLEY RAFTER TO RIDGE BEAM WITH (3) 16d (3.5" X 0.131") TOE-NAILS. FASTEN VALLEY RAFTER TO VALLEY PLATE WITH (3) 16d (3.5" X 0.131") TOE-NAILS.
- (7) SUPPORT THE VALLEY RAFTERS WITH 2 X 4 POSTS AT 48" O/C (OR LESS) ALONG EACH RAFTER. INSTALL POSTS IN A STAGGERED PATTERN AS SHOWN ON PLAN DRAWING. ALIGN POSTS WITH TRUSSES BELOW. FASTEN VALLEY RAFTER TO POST WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST THROUGH SHEATHING TO SUPPORTING TRUSSES WITH (2) 16d (3.5" X 0.131") NAILS.
- (8) POSTS SHALL BE 2 X 4 #2 SPF OR BETTER. POSTS EXCEEDING 75" IN HEIGHT SHALL BE INCREASED TO 4 X 4 #2 SPF, OR BETTER, OR BE PRE-ASSEMBLED TWO (2) PLY 2 X 4 #2 SPF OR BETTER FASTENED TOGETHER WITH 2 ROWS OF 10d (3" X 0.131") NAILS AT 6" O/C.
- (9) MAINTAIN A MINIMUM 3/4" LUMBER EDGE DISTANCE WHEN NAILING. NAIL SPACING SHOULD APPROXIMATE A MINIMUM 1-3/4" O/C OR MORE UNLESS NOTED OTHERWISE. ALL CONSTRUCTION TO CONFORM TO ONTARIO BUILDING CODE (CURRENT ADDITION) AT ALL TIMES.

NOTES:

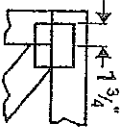
- (10) 48" O/C (MAXIMUM POST SPACING).
- (11) ROOF LIVE LOAD = 34.8 PSF (MAX.).
- (12) ROOF DEAD LOAD = 10.0 PSF (MAX.).
- (13) PART 9 APPLICATION ONLY (ONTARIO BUILDING CODE)
- (14) PART 4 APPLICATION ONLY (ONTARIO BUILDING CODE) WITH APPROVED REVIEW BY LICENSED PROFESSIONAL ENGINEER.
- (15) BASE TRUSS SPACING (24" O/C MAX.).
- (16) ALL PRE-ENGINEERED BASE TRUSS COMPONENTS TO BE SEALED BY LICENSED PROFESSIONAL ENGINEER AND THIS DETAIL TO BE VERIFIED AND APPROVED BY SAME WHEN RIDGE BOARD LENGTH EXCEEDS 12'-0".
- (17) ALL BASE TRUSSES: P = 4 (4/12) - MINIMUM.
- (18) ALL VALLEY RAFTERS: P = 4 (4/12) - MINIMUM.



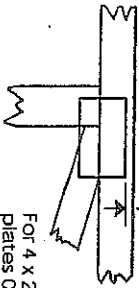
AWA NO TAW 6305.14
STRUCTURAL
COMPONENT ONLY

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless X, Y offsets are indicated. Dimensions are in feet-inches-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0 1/4" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

* Plate location details available in Mitek software or upon request.

PLATE SIZE

4 X 4

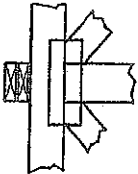
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use 1, I or Eliminator bracing if indicated.

BEARING

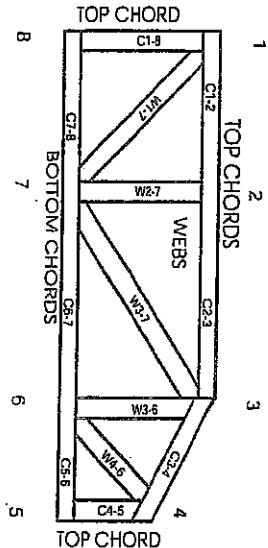


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:
TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DS8-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in feet-inches-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

T1996-L, 10319-L, 13270-L, 12691-R

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General Safety Notes

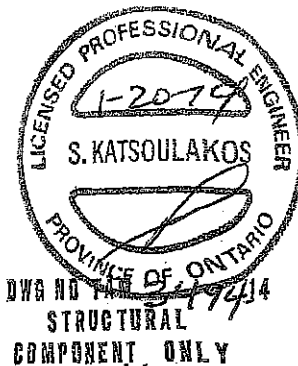
Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative I, L, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

Micro City Engineering Services Inc.
(BCIN: 26064; FIRM BCIN: 29991)

RR #1, Po Box 61
Glencoe, Ontario
N0L 1M0

(519) 287 - 2242; Fax: (519) 287 - 5750 (Call)



Responsibilities:

Micro City Engineering Services is responsible for the design of trusses as individual components.

It is the responsibilities of others to ascertain that the design loads utilized on this (these) drawing(s) meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdiction over such decisions.

All dimensions are to be verified by the owner, contractor, architect, or other authority having input over such decisions prior to truss component manufacture. At no time shall Micro City Engineering Services Inc. or its employees be responsible for dimension errors.

Micro City Engineering Services Inc. bears no responsibility for the erection of any truss components. Persons erecting truss components are cautioned to seek professional advice regarding temporary and permanent bracing systems and to be totally familiar with all aspects of truss erection prior to proceeding on any truss component erection job. Any bracing shown on Micro City Engineering Services Inc. or Tamarack Roof Trusses Inc. sealed or unsealed truss component drawings is specified for the single truss component in question and is identified as an integral part of the design for that particular truss component but is not meant to represent the only required bracing for that particular truss component when installed as a component in a series of truss components in a roof truss system.

It is the truss manufacturer's responsibility to ensure that trusses are manufactured in accordance with Micro City Engineering Services Inc. specifications outlined below:

SPECIFICATIONS:

Truss components sealed by Micro City Engineering Services Inc. must conform to the relevant sections of the current Building Code of Ontario and Canada (Part 4 or Part 9) or the current Farm Building Code of Canada in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the Truss Plate Institute of Canada (TPIC). All unit lumber and nailing stresses identified on truss component design drawings and/or used in the design of individual truss components shall conform to the current CSA Wood Design standard identified in the current Building Code and TPIC Design Standards.

The lumber used to manufacture any truss component is to conform to the specified size and grade identified on the truss drawing.

The lumber used in the manufacture of any truss component is not to exceed 19% during its service use unless specifically noted on the truss drawing.

The lumber used in the manufacture of any truss component is not to be treated with any chemicals during its service life unless specifically noted on the truss drawing.

Connector plates shall be applied to both faces of the truss component at each joint and shall be positioned exactly as specified.

The top chord of any truss component is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the sealed truss component drawing but not exceeding 24" o/c (Part 9 design) and not exceeding 48" o/c (Part 4 or Agricultural design).

When a truss component is to be installed with no rigid ceiling attached directly to the bottom chord, then the bottom chord is to be laterally braced at intervals not exceeding 3m (or 10'-0").

All sealed or unsealed truss component drawings provided by Micro City Engineering Services Inc. Or Tamarack Roof Trusses Inc. should be read in conjunction with the following:

Warning-Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473C rev 10-'08 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer - not the truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component Safety Information available from the Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA, 22314.