ASPHALT SHINGLES
12"FINISH O.H.
R.T.M.C
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:

HGUS26-2 -(XX)
LJS26DS -(V)
LUS24 -(O)

8/12 PITCHES (TYP.)
UNLESS NOTED

DENOTES:

* -1'-6"-0 HIGHER PLATE

H -2'-0-0 HIGHER PLATE

ALL B- 2-2X10 BEAMS (FLUSH)

T-170534

DENOTES:
CONVENTIONAL
FRAMING

HIGHGROVE-5E
EL:2 - UNIT1

285953

HIGHGROVE-3
EL:2 - UNIT2

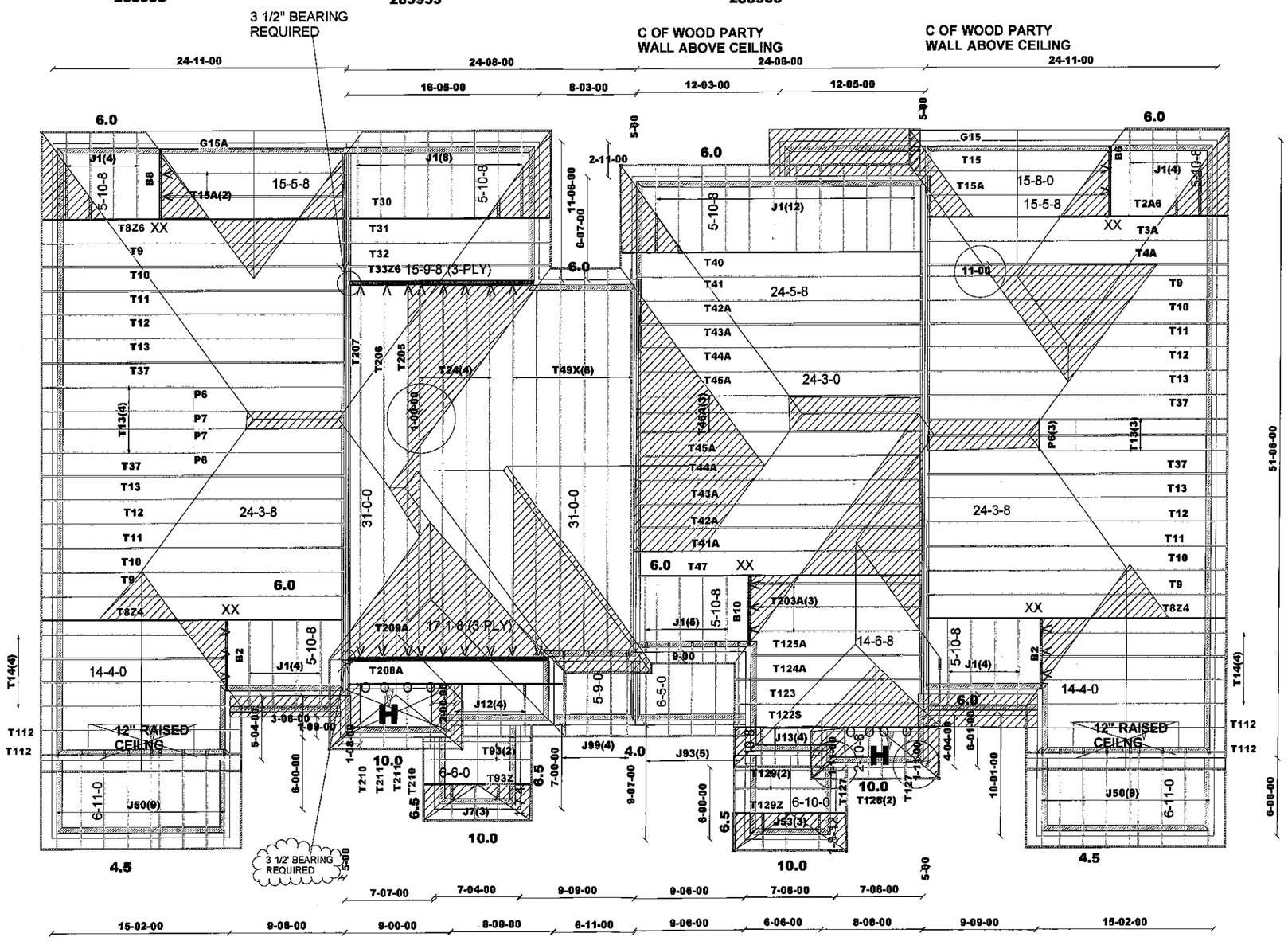
285955

HIGHGROVE-4
EL:2 (REV) - UNIT3

285956

HIGHGROVE-5E
EL:2(REV) - UNIT4

285957



ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION
ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"o.c.
WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.
POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE
BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN LOADS:

SNOW LOAD	25.6 PSF
TC DEAD	3 PSF
BC LIVE	10.5 PSF
BC DEAD	7 PSF

m11,007

TAMARACK LUMBER INC. ALPA LUMBER GROUP

Job Track: 43954	Builder / Location: GREENPARK HOMES / WATERDOWN	Model / Elevation: BLOCK210/ UNIT1TO4 (REVISED)	Mitek ver 7.5.0
Layout ID: 285952			
Plan Log: 93446	Project: RUSSEL GARDENS PHASE 2	THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTLILZED FOR ANY OTHER PURPOSE.	
	Date: 4/18/2018 Designer: sonny		

TAMARACK LUMBER INC
ALPA LUMBER GROUP

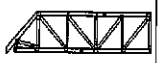
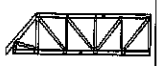
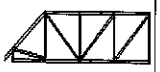
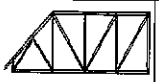
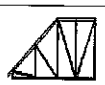
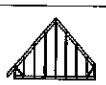
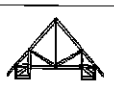
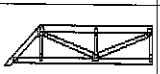
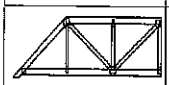
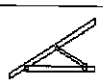
Delivery Shiplist

DATE	04/18/18
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 285953 LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH 2 SUB-BUILDER:
MODEL: HIGHGROVE 5E ELEVATION: 2- UNIT1

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T8Z4 HALF HIP	8.00 0.00	24-03-08	04-01-04	2 X 6	2 X 6	01-03-08 00-00-00	01-04-13 04-01-04	143.26 88.34		
	1	T8Z6 HALF HIP	8.00 0.00	24-03-08	04-01-04	2 X 6	2 X 6	01-03-08 00-00-00	01-04-13 04-01-04	143.26 88.34		
	2	T9 HALF HIP	8.00 0.00	24-03-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-01-04	197.10 123.66		
	2	T10 HALF HIP	8.00 0.00	24-03-08	06-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 06-01-04	212.66 133.34		
	2	T11 HALF HIP	8.00 0.00	24-03-08	07-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 07-01-04	225.56 142.68		
	2	T12 HALF HIP	8.00 0.00	24-03-08	08-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 08-01-04	234.64 146.34		
	6	T13 HALF HIP	8.00 0.00	24-03-08	09-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 09-01-04	737.28 454.98		
	2	T37 HALF HIP	8.00 0.00	24-03-08	10-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 10-01-04	257.54 159.34		
	2	T15A COMMON	8.00 0.00	15-05-08	06-07-08	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	129.00 82.66		
	1	G15A COMMON	8.00 0.00	15-05-08	06-07-08	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	67.52 44.33		
	4	T14 COMMON	8.00 0.00	14-04-00	06-02-02	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	247.04 156.00		
	2	T112 ROOF	8.00 0.00	14-04-00	06-02-02	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	135.80 89.00		
	2	P6 PIGGYBACK	8.00 0.00	11-11-15	01-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-00-00 02-00-00	80.22 51.34		
	2	P7 PIGGYBACK	8.00 0.00	11-11-15	02-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-00-00 03-00-00	85.52 56.00		
	8	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	134.32 85.36		
	9	J50 JACK	4.50 0.00	06-11-00	02-11-12	2 X 4	2 X 4	01-03-08 00-00-00	00-04-10 02-11-12	184.86 114.03		

TOTAL # TRUSS= **48.00** TOTAL BFT OF ALL TRUSSES= **2015.74** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3215.58** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16

TAMARACK LUMBER INC
ALPA LUMBER GROUP

Delivery Shiplist

DATE	04/18/18
SALES REP	Mario

JOB TRACK: 43954 **LAYOUT ID:** 285953 **LOCATION:**
BUILDER: GREENPARK - RUSSELL GARDENS PH2 **SUB-BUILDER:**
MODEL: HIGHGROVE 5E **ELEVATION:** 2- UNIT1

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
5	Hangers	LJS26DS	

TOTAL # ITEMS= **7.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

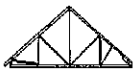
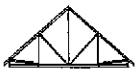
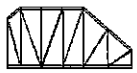
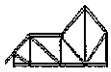
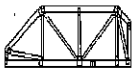
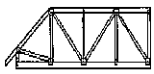
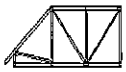
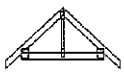
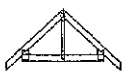
Delivery Shiplist

DATE	09/08/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 285955 LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH2 SUB-BUILDER:
MODEL: HIGHGROVE 3 ELEVATION: 2- UNIT2

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	6	T49X COMMON	6.00 0.00	31-00-00	08-08-12	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 00-09-08	717.12 444.00		
	4	T24 COMMON	6.00 0.00	31-00-00	08-11-00	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	486.20 306.00		
	1	T205 HIP	6.00 0.00	31-00-00	08-06-08	2 X 4	2 X 4	00-00-00 00-00-00	06-11-00 02-07-00	152.77 96.17		
	1	T206 HIP	6.00 0.00	31-00-00	09-10-08	2 X 4	2 X 4	00-00-00 00-00-00	06-11-00 02-07-00	155.22 97.50		
	1	T207 HIP	6.00 0.00	31-00-00	11-02-08	2 X 4	2 X 4	00-00-00 00-00-00	06-11-00 02-07-00	161.32 100.67		
	1	T208A ROOF	8.00 0.00	17-01-08	06-03-02	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 03-06-08	73.08 47.83		
	1 3 Ply	T209A HIP GIRDER	8.00 0.00	17-01-08	04-08-08	2 X 6	2 X 6	00-00-00 00-00-00	01-04-13 03-06-08	323.25 202.02		
	1	T30 HALF HIP	8.00 0.00	15-09-08	04-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-04-13 04-01-04	77.31 47.50		
	1	T31 HALF HIP	8.00 0.00	15-09-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-01-04	67.94 43.00		
	1	T32 HALF HIP	8.00 0.00	15-09-08	06-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 06-01-04	74.63 46.67		
	1 3 Ply	T33Z6 HALF HIP	8.00 0.00	15-09-08	06-08-12	2 X 6	2 X 6	00-00-00 00-00-00	01-04-13 06-08-12	341.19 201.00		
	2	T210 HALF HIP	10.00 0.00	03-11-08	02-02-13	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 04-02-13	43.30 29.34		
	2	T211 HALF HIP	10.00 0.00	03-11-08	03-06-13	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 05-06-13	52.08 35.34		
	2	T93 COMMON	6.50 0.00	06-06-00	02-04-09	2 X 4	2 X 4	01-03-08 01-03-08	00-07-07 00-07-07	44.40 30.00		
	1	T93Z COMMON	6.50 0.00	06-06-00	02-04-09	2 X 4	2 X 4	01-03-08 01-03-08	00-07-07 00-07-07	22.20 15.00		
	8	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	134.32 85.36		
	3	J7 JACK-OPEN	10.00 0.00	01-07-04	02-04-07	2 X 4	2 X 4	01-03-08 00-00-00	01-00-06 02-04-07	21.27 15.99		
	4	J12 JACK-OPEN	8.00 0.00	02-11-08	03-04-08	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 03-04-08	42.04 26.68		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

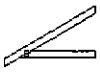
Delivery Shiplist

DATE	09/08/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 285955 LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH2 SUB-BUILDER:
MODEL: HIGHGROVE 3 ELEVATION: 2- UNIT2

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	4	J99 MONOPITCH	4.00 / 0.00	05-09-00	02-02-15	2 X 4	2 X 4	01-03-08 / 00-00-00	00-03-15 / 02-02-15	60.52 / 37.32		

TOTAL # TRUSS= **49.00** TOTAL BFT OF ALL TRUSSES= **1907.39** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3050.16** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
17	Hangers	LJS26DS	
4	Hangers	LUS24	

TOTAL # ITEMS= **21.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

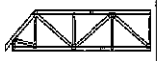
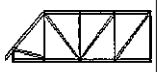
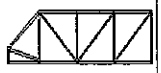
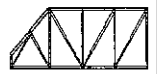
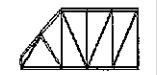
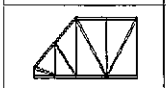
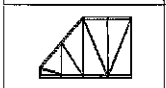
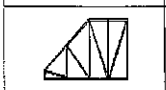
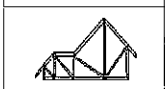
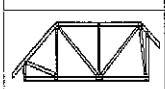
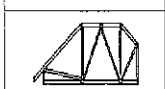
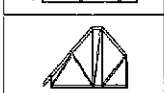
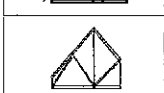
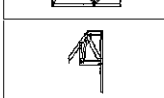
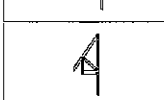
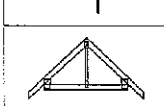
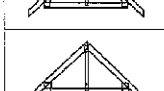
Delivery Shiplist

DATE	09/08/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 285956 LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH2 SUB-BUILDER:
MODEL: HIGHGROVE4 ELEVATION: 2 (REV)- UNIT3

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T40 HALF HIP	8.00 0.00	24-05-08	04-01-04	2 X 6	2 X 6	01-03-08 00-00-00	01-04-13 04-01-04	136.90 84.33		
	1	T41 HALF HIP	8.00 0.00	24-05-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-01-04	99.05 61.83		
	1	T41A HALF HIP	8.00 0.00	24-03-00	05-01-04	2 X 4	2 X 4	00-00-00 00-00-00	01-06-08 05-01-04	96.77 61.17		
	2	T42A HALF HIP	8.00 0.00	24-03-00	06-01-04	2 X 4	2 X 4	00-00-00 00-00-00	01-06-08 06-01-04	209.24 132.00		
	2	T43A HALF HIP	8.00 0.00	24-03-00	07-01-04	2 X 4	2 X 4	00-00-00 00-00-00	01-06-08 07-01-04	222.10 141.34		
	2	T44A HALF HIP	8.00 0.00	24-03-00	08-01-04	2 X 4	2 X 4	00-00-00 00-00-00	01-06-08 08-01-04	231.32 146.34		
	2	T45A HALF HIP	8.00 0.00	24-03-00	09-01-04	2 X 4	2 X 4	00-00-00 00-00-00	01-06-08 09-01-04	242.42 150.34		
	3	T46A HALF HIP	8.00 0.00	24-03-00	10-01-04	2 X 4	2 X 4	00-00-00 00-00-00	01-06-08 10-01-04	381.24 237.00		
	1	T47 FLAT GIRDER	0.00 0.00	24-03-00	04-01-04	2 X 6	2 X 6	00-00-00 00-00-00	04-01-04 04-01-04	145.78 90.67		
	1	T122S ROOF	8.00 0.00	14-09-00	06-00-07	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 03-04-13	65.51 41.84		
	1	T123 HIP	8.00 0.00	14-09-08	03-11-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 03-04-13	64.00 40.00		
	1	T124A HIP	8.00 0.00	14-06-08	05-03-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 03-06-08	66.41 42.00		
	1	T125A HIP	8.00 0.00	14-06-08	06-07-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 03-06-08	70.54 46.17		
	3	T203A COMMON	8.00 0.00	14-06-08	07-03-13	2 X 4	2 X 4	00-00-00 00-00-00	01-04-13 03-06-08	183.60 117.00		
	2	T127 HALF HIP	10.00 0.00	02-10-08	02-04-13	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 04-04-13	38.22 27.34		
	2	T128 HALF HIP	10.00 0.00	02-10-08	03-08-13	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 05-08-13	37.94 25.66		
	2	T129 COMMON	6.50 0.00	06-10-00	02-05-10	2 X 4	2 X 4	01-03-08 01-03-08	00-07-07 00-07-07	46.18 30.00		
	1	T129Z COMMON	6.50 0.00	06-10-00	02-05-10	2 X 4	2 X 4	01-03-08 01-03-08	00-07-07 00-07-07	23.09 15.00		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

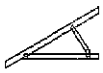
Delivery Shiplist

DATE	09/08/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 285956 LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH 2 SUB-BUILDER:
MODEL: HIGHGROVE4 ELEVATION: 2 (REV)- UNIT3

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	17	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	285.43 181.39		
	4	J13 JACK-OPEN	8.00 0.00	01-10-08	02-07-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 02-07-13	31.16 18.68		
	3	J53 JACK-OPEN	10.00 0.00	01-08-12	02-05-11	2 X 4	2 X 4	01-03-08 00-00-00	01-00-06 00-05-13	22.26 15.99		
	5	J93 MONOPITCH	4.00 0.00	06-05-00	02-05-10	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-05-10	90.80 58.35		

TOTAL # TRUSS= **58.00** TOTAL BFT OF ALL TRUSSES= **1764.44** BFT. TOTAL WEIGHT OF ALL TRUSSES= **2789.96** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	HGUS26-2	
3	Hangers	LJS26DS	
4	Hangers	LUS24	

TOTAL # ITEMS= **8.00**

TAMARACK LUMBER INC
ALPA LUMBER GROUP

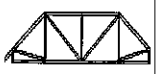
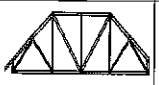
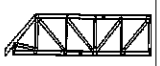
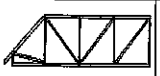
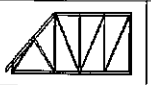
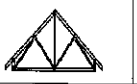
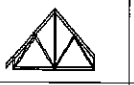
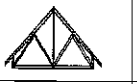
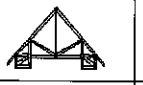
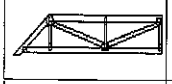
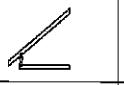
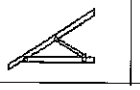
Delivery Shiplist

DATE	04/18/18
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 285957 LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH 2 SUB-BUILDER:
MODEL: HIGHGROVE 5E ELEVATION: 2(REV)- UNIT4

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T2A6 HIP GIRDER	8.00 0.00	24-03-08	04-01-04	2 X 6	2 X 6	01-03-08 00-00-00	01-04-13 01-06-08	142.19 87.67		
	1	T3A HIP	8.00 0.00	24-03-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	94.81 60.17		
	1	T4A HIP	8.00 0.00	24-03-08	06-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	103.53 66.33		
	1	T8Z4 HALF HIP	8.00 0.00	24-03-08	04-01-04	2 X 6	2 X 6	01-03-08 00-00-00	01-04-13 04-01-04	143.26 88.34		
	2	T9 HALF HIP	8.00 0.00	24-03-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-01-04	197.10 123.66		
	2	T10 HALF HIP	8.00 0.00	24-03-08	06-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 06-01-04	212.66 133.34		
	2	T11 HALF HIP	8.00 0.00	24-03-08	07-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 07-01-04	225.56 142.68		
	2	T12 HALF HIP	8.00 0.00	24-03-08	08-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 08-01-04	234.64 146.34		
	5	T13 HALF HIP	8.00 0.00	24-03-08	09-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 09-01-04	614.40 379.15		
	2	T37 HALF HIP	8.00 0.00	24-03-08	10-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 10-01-04	257.54 159.34		
	1	T15 COMMON	8.00 0.00	15-08-00	06-07-08	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	66.76 42.00		
	1	G15 COMMON	8.00 0.00	15-08-00	06-07-08	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	69.72 45.00		
	1	T15A COMMON	8.00 0.00	15-05-08	06-07-08	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	64.50 41.33		
	4	T14 COMMON	8.00 0.00	14-04-00	06-02-02	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	247.04 156.00		
	2	T112 ROOF	8.00 0.00	14-04-00	06-02-02	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	135.80 89.00		
	3	P6 PIGGYBACK	8.00 0.00	11-11-15	01-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-00-00 02-00-00	120.33 77.01		
	8	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	134.32 85.36		
	9	J50 JACK	4.50 0.00	06-11-00	02-11-12	2 X 4	2 X 4	01-03-08 00-00-00	00-04-10 02-11-12	184.86 114.03		

TOTAL # TRUSS= **48.00** TOTAL BFT OF ALL TRUSSES= **2036.75** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3249.02** LBS.

TAMARACK LUMBER INC
ALPA LUMBER GROUP

Delivery Shiplist

DATE	04/18/18
SALES REP	Mario

JOB TRACK: 43954 **LAYOUT ID:** 285957 **LOCATION:**
BUILDER: GREENPARK - RUSSELL GARDENS PH2 **SUB-BUILDER:**
MODEL: HIGHGROVE 5E **ELEVATION:** 2(REV)- UNIT4

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
5	Hangers	LJS26DS	

TOTAL # ITEMS= **7.00**

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285957	T2A6	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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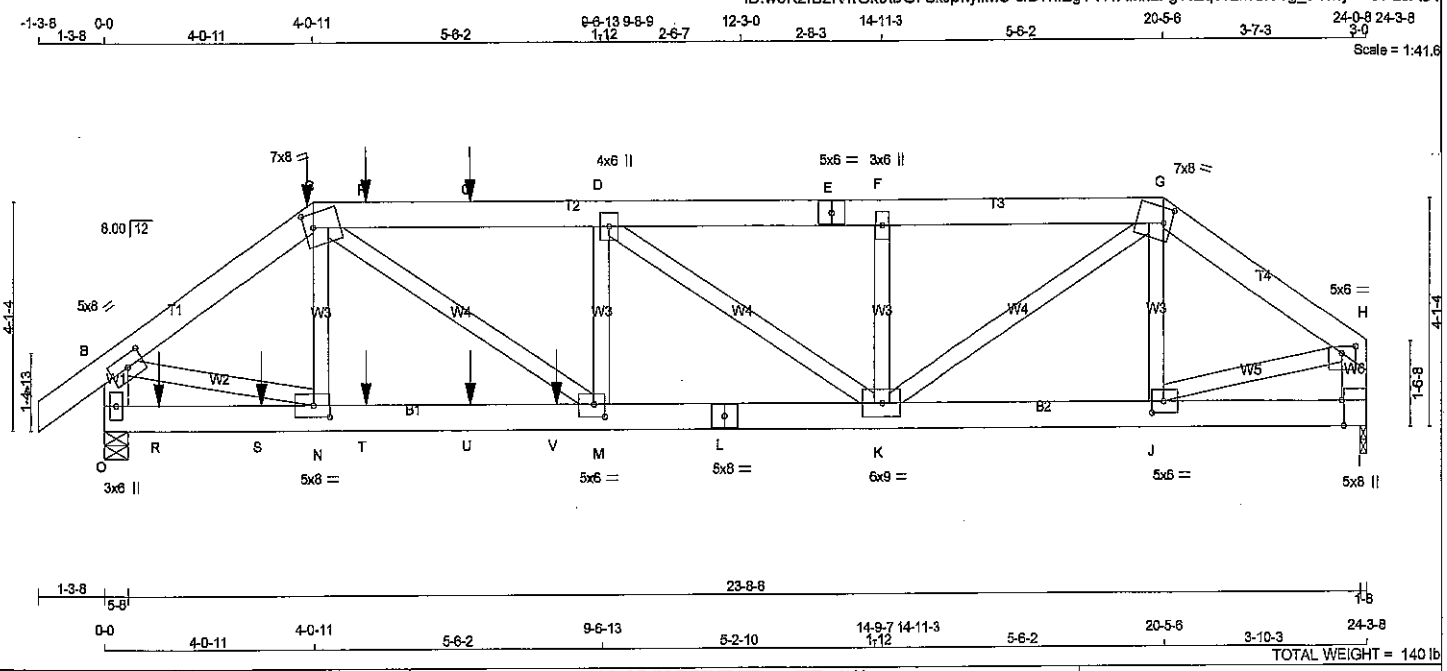

TOTAL WEIGHT = 140 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - G	2x6	DRY	No.2	SPF
G - H	2x6	DRY	No.2	SPF
O - B	2x6	DRY	No.2	SPF
I - H	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.50	3.75
C	TTWW-m	MT20	7.0	8.0	3.00	2.00
D	TMWW+t	MT20	4.0	6.0		
E	TS-t	MT20	5.0	6.0		
F	TMW+w	MT20	3.0	6.0		
G	TTWW-m	MT20	7.0	8.0	3.25	1.75
H	TMVW-p	MT20	5.0	6.0	1.50	3.00
I	BMV1+p	MT20	5.0	8.0	Edge	0.50
J	BMWW-t	MT20	5.0	6.0	2.50	2.75
K	BMWWW-t	MT20	6.0	9.0		
L	BS-t	MT20	5.0	6.0		
M	BMWW-t	MT20	5.0	6.0	2.75	2.50
N	BMWW-t	MT20	5.0	8.0	2.50	3.75
O	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 101.4 lbs FACTORED DOWN AT 5-0-12, AND 101.4 lbs FACTORED DOWN AT 7-0-12, AND 244.5 lbs FACTORED DOWN AT 4-0-11 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 1-0-12, 69.9 lbs FACTORED DOWN AT 3-0-12, 69.9 lbs FACTORED DOWN AT 5-0-12, AND 69.9 lbs FACTORED DOWN AT 7-0-12, AND 1158.5 lbs FACTORED DOWN AT 8-8-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	2715	0	2715	0	0	5-8	5-8
I	1897	0	1897	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	2210	1258 / 0	483 / 0	0 / 0	0 / 0	470 / 0	0 / 0
I	1551	871 / 0	347 / 0	0 / 0	0 / 0	333 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.69 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 33	-84.3	-84.3	0.07 (1)	10.00	N- C	-111 / 265	0.05 (3)
B- C	-3253 / 0	-84.3	-84.3	0.19 (1)	4.52	C- M	0 / 2136	0.36 (1)
C- P	-4480 / 0	-84.3	-84.3	0.43 (1)	3.69	M- D	0 / 263	0.05 (3)
P- Q	-4480 / 0	-84.3	-84.3	0.43 (1)	3.69	K- F	-468 / 0	0.08 (1)
Q- D	-4480 / 0	-84.3	-84.3	0.43 (1)	3.69	K- G	0 / 2076	0.37 (1)
D- E	-3550 / 0	-84.3	-84.3	0.26 (1)	4.30	J- G	-288 / 35	0.05 (1)
E- F	-3550 / 0	-84.3	-84.3	0.26 (1)	4.30	B- N	0 / 2761	0.49 (1)
F- G	-3550 / 0	-84.3	-84.3	0.26 (1)	4.29	J- H	0 / 1887	0.33 (1)
G- H	-2201 / 0	-84.3	-84.3	0.14 (1)	5.34	D- K	-1125 / 0	0.58 (1)
O- B	-2682 / 0	0.0	0.0	0.19 (1)	6.32			
I- H	-1864 / 0	0.0	0.0	0.13 (1)	7.29			
O- R	0 / 0	-28.0	-28.0	0.23 (2)	10.00			
R- S	0 / 0	-28.0	-28.0	0.23 (2)	10.00			
S- N	0 / 0	-28.0	-28.0	0.23 (2)	10.00			
N- T	0 / 2695	-28.0	-28.0	0.78 (1)	10.00			
T- U	0 / 2695	-28.0	-28.0	0.78 (1)	10.00			
U- V	0 / 2695	-28.0	-28.0	0.78 (1)	10.00			
V- M	0 / 2695	-28.0	-28.0	0.78 (1)	10.00			
M- L	0 / 4480	-28.0	-28.0	0.83 (1)	10.00			
L- K	0 / 4480	-28.0	-28.0	0.83 (1)	10.00			
K- J	0 / 1815	-26.0	-28.0	0.32 (1)	10.00			
J- I	0 / 0	-28.0	-28.0	0.09 (2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	4-0-11	-23	-25	---	FRONT	VERT	DEAD
C	4-0-11	-222	-222	---	FRONT	VERT	SNOW
P	5-0-12	-101	-101	---	FRONT	VERT	TOTAL
Q	7-0-12	-101	-101	---	FRONT	VERT	TOTAL
R	1-0-12	-40	-70	---	FRONT	VERT	TOTAL
S	3-0-12	-40	-70	---	FRONT	VERT	TOTAL
T	5-0-12	-40	-70	---	FRONT	VERT	TOTAL
U	7-0-12	-40	-70	---	FRONT	VERT	TOTAL
V	8-8-8	-1158	-1158	---	FRONT	VERT	TOTAL

[M]

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	25.8	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.23")

CSI: TC=0.43/1.00 (C-D:1) , BC=0.83/1.00 (K-M:1) , WB=0.58/1.00 (D-K:1) , SSI=0.72/1.00 (M-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (G) (INPUT = 0.90)
JSI METAL= 0.71 (L) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 5110 -18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285862	T3A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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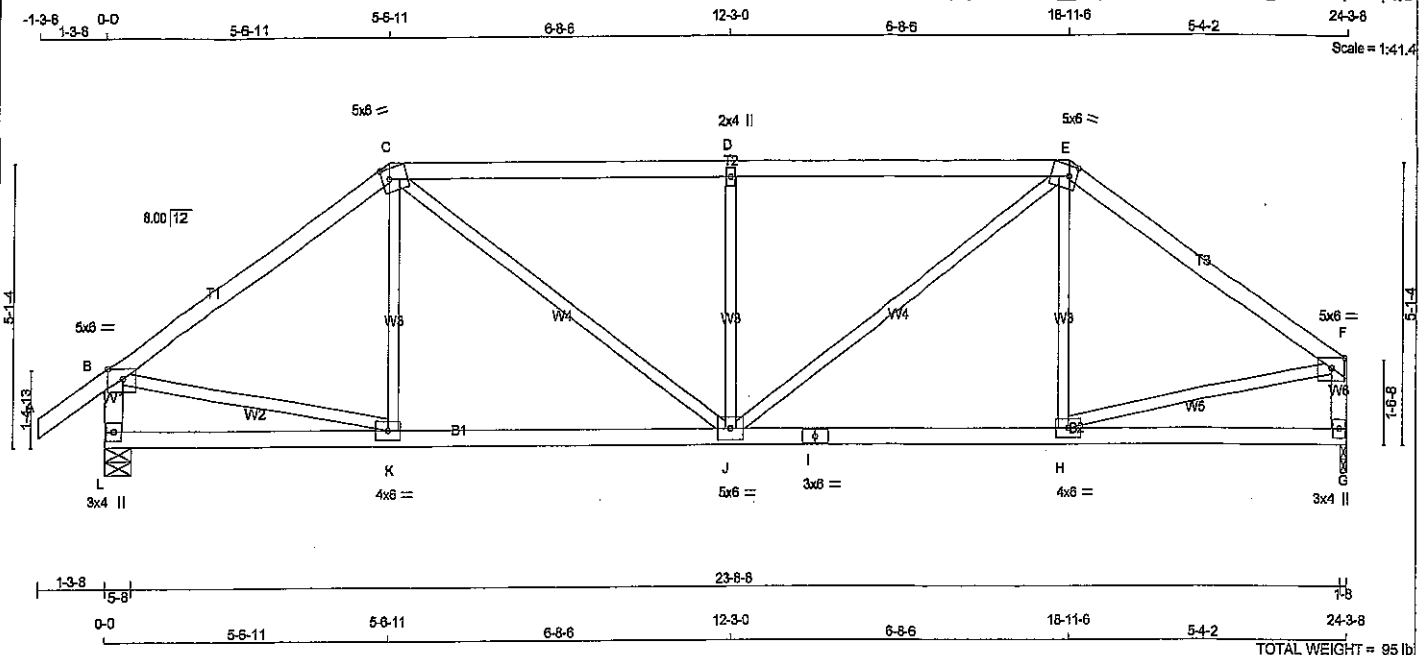


TOTAL WEIGHT = 95 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW-p	MT20	5.0	6.0	Edge	
C	TTWW-m	MT20	5.0	6.0	2.25	1.75
D	TMW+w	MT20	2.0	4.0		
E	TTWW-m	MT20	5.0	6.0	2.25	1.75
F	TMW-p	MT20	5.0	6.0	Edge	
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	5.0	6.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1479	0	1479	0	0	5-8	5-8
G	1363	0	1363	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1198	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0
G	1119	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0

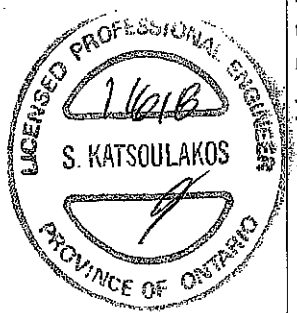
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	K-C	-45 / 211	0.05 (3)
B-C	-1485 / 0	-84.3	-84.3	0.55 (1)	4.69	C-J	0 / 728	0.16 (1)
C-D	-1833 / 0	-84.3	-84.3	0.80 (1)	3.88	J-D	-692 / 0	0.27 (1)
D-E	-1833 / 0	-84.3	-84.3	0.80 (1)	3.88	J-E	0 / 769	0.17 (1)
E-F	-1456 / 0	-84.3	-84.3	0.50 (1)	4.81	H-E	-81 / 187	0.04 (3)
L-B	-1418 / 0	0.0	0.0	0.15 (1)	6.65	B-K	0 / 1265	0.28 (1)
G-F	-1305 / 0	0.0	0.0	0.14 (1)	7.06	H-F	0 / 1239	0.28 (1)
L-K	0 / 0	-28.0	-28.0	0.25 (3)	10.00			
K-J	0 / 1241	-28.0	-28.0	0.40 (2)	10.00			
J-I	0 / 1208	-28.0	-28.0	0.39 (2)	10.00			
I-H	0 / 1208	-28.0	-28.0	0.39 (2)	10.00			
H-G	0 / 0	-28.0	-28.0	0.24 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.60/1.00 (D-E:1) , BC=0.40/1.00 (J-K:2) , WB=0.28/1.00 (B-K:1) , SSI=0.27/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LDAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (E) (INPUT = 0.90)
JSI METAL= 0.35 (I) (INPUT = 1.00)

DWG NO. TAM 4008-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285862	T4A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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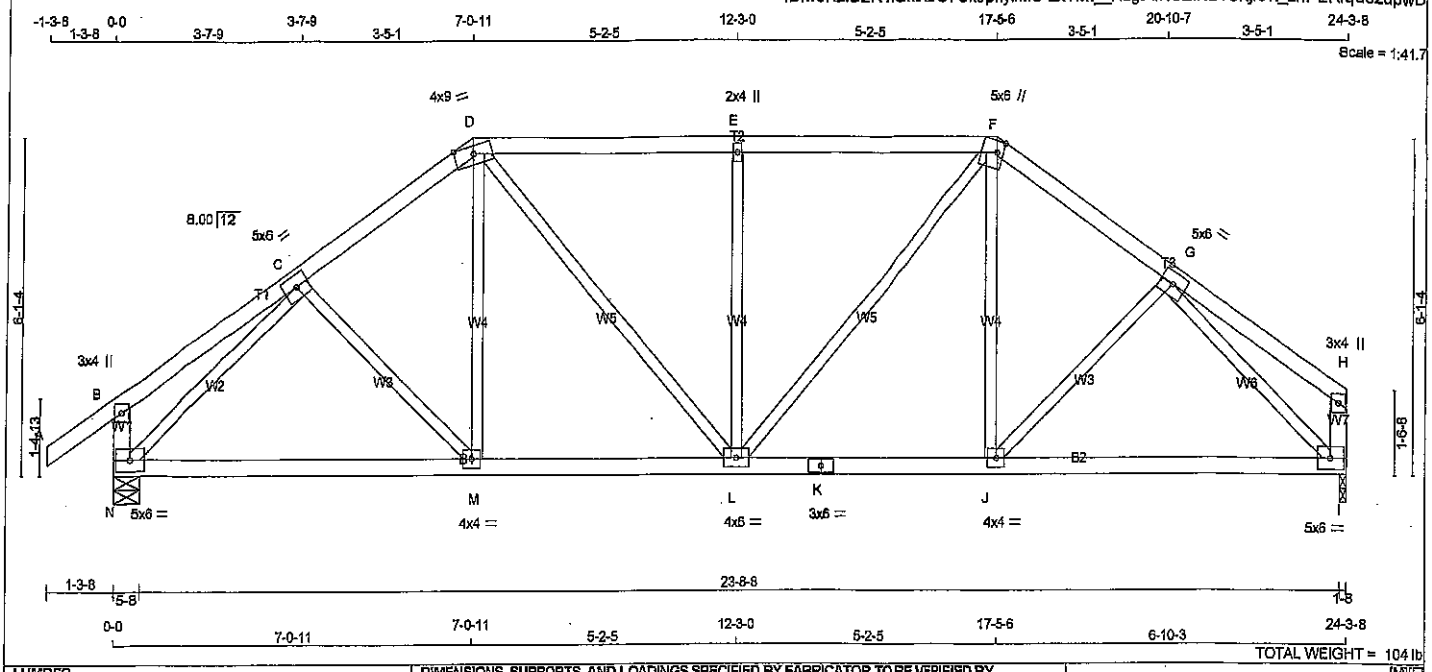


TOTAL WEIGHT = 104 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW-m	MT20	4.0	9.0	Edge	
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.50	1.50
G	TMWW-t	MT20	5.0	6.0		
H	TMV+p	MT20	3.0	4.0		
I	BMWW1-t	MT20	5.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	4.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMWW1-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
N	1479	0	1479	0	0	5-8	5-8
I	1363	0	1363	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1198	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0
I	1119	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, I

BRACING

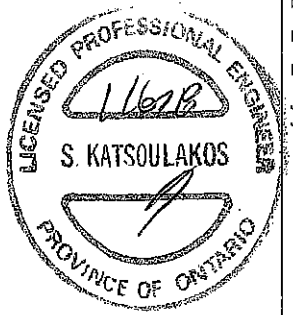
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.05 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	C- M	0 / 117	0.03 (3)
B- C	0 / 17	-84.3	-84.3	0.15 (1)	10.00	M- D	0 / 305	0.07 (2)
C- D	-1459 / 0	-84.3	-84.3	0.14 (1)	5.29	D- L	0 / 409	0.09 (1)
D- E	-1474 / 0	-84.3	-84.3	0.31 (1)	5.05	L- E	-534 / 0	0.31 (1)
E- F	-1474 / 0	-84.3	-84.3	0.31 (1)	5.05	L- F	0 / 442	0.10 (1)
F- G	-1433 / 0	-84.3	-84.3	0.14 (1)	5.33	J- F	0 / 273	0.06 (2)
G- H	0 / 17	-84.3	-84.3	0.14 (1)	10.00	J- G	0 / 138	0.03 (3)
N- B	-233 / 0	0.0	0.0	0.02 (1)	7.81	N- C	-1659 / 0	0.64 (1)
I- H	-106 / 0	0.0	0.0	0.01 (1)	7.81	G- I	-1635 / 0	0.59 (1)
N- M	0 / 1196	-28.0	-28.0	0.45 (2)	10.00			
M- L	0 / 1201	-28.0	-28.0	0.46 (2)	10.00			
L- K	0 / 1179	-28.0	-28.0	0.45 (2)	10.00			
K- J	0 / 1179	-28.0	-28.0	0.45 (2)	10.00			
J- I	0 / 1136	-28.0	-28.0	0.43 (2)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.31/1.00 (D-E:1) , BC=0.46/1.00 (L-M:2) , WB=0.64/1.00 (C-N:1) , SSI=0.21/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (D) (INPUT = 0.90)
JSI METAL= 0.41 (C) (INPUT = 1.00)

DWG NO. TAM 4009-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43994	DRWG NO.
285813	T8Z4	1	1	TRUSS DESC.	

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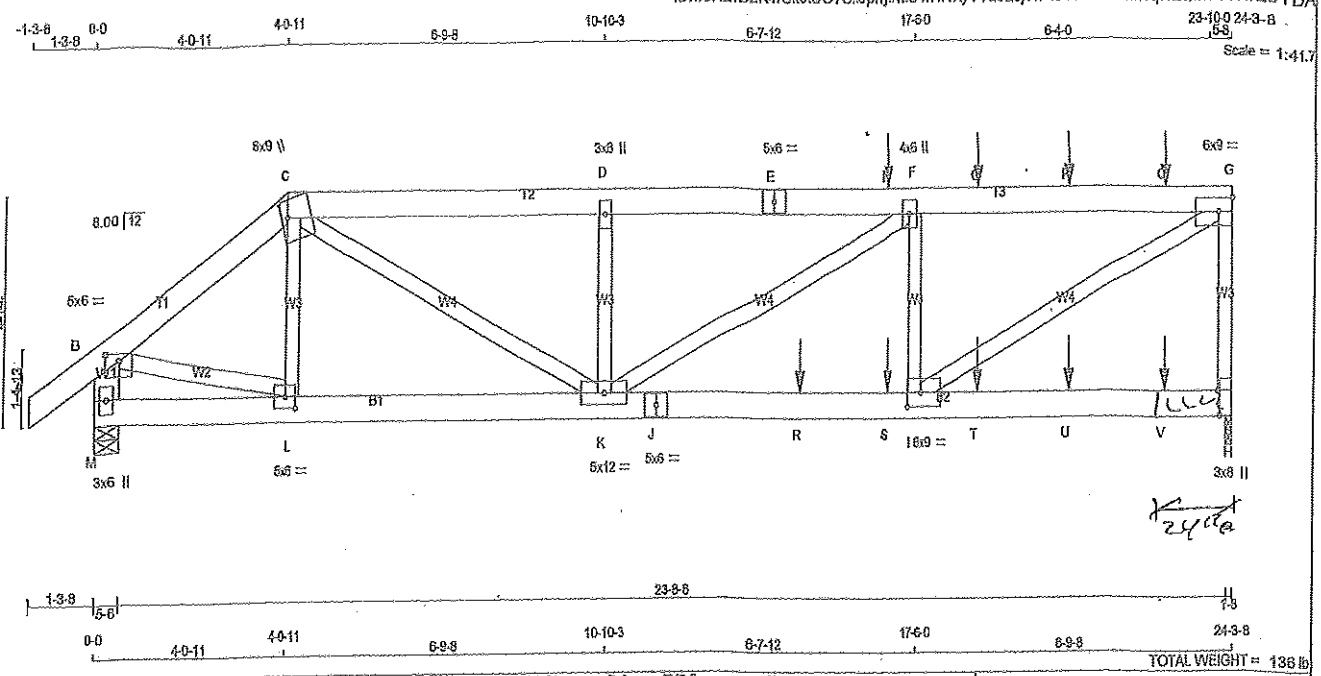


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - G	2x6	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x6	DRY	No.2	SPF
M - J	2x6	DRY	2100F 1.8E	SPF
J - H	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW-p	MT20	5.0	8.0	1.50	3.00
C	TTWW+m	MT20	8.0	9.0	3.75	1.75
D	TMW+w	MT20	3.0	6.0		
E	TS-t	MT20	5.0	6.0		
F	TMWW+t	MT20	4.0	6.0		
G	TMW-t	MT20	6.0	9.0		Edge
H	BMV1+p	MT20	3.0	8.0	5.50	0.50
I	BMWW-t	MT20	6.0	9.0	3.00	3.75
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	5.0	12.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.75
M	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 101.4 lbs FACTORED DOWN AT 16-10-12, 101.4 lbs FACTORED DOWN AT 18-10-12, AND 101.4 lbs FACTORED DOWN AT 20-10-12, AND 101.4 lbs FACTORED DOWN AT 22-10-12 ON TOP CHORD, AND 1340.4 lbs FACTORED DOWN AT 14-11-8, 69.9 lbs FACTORED DOWN AT 16-10-12, 69.9 lbs FACTORED DOWN AT 18-10-12, AND 69.9 lbs FACTORED DOWN AT 20-10-12, AND 69.9 lbs FACTORED DOWN AT 22-10-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

SCAB JOINT # H & ~~I~~ WITH 2 X 6 #2 SPF 24" LONG / SIDE(S) USING 2 ROW(S) 3¼" ARDOX SPIRAL/~~COMMON WIRE~~ NAILS AT 2" O/C STAGGERED (12 NAILS/SCAB). REQUIRED FOR 1/4 SRG PROVISION.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	2652	0	2652	0	0	1-8	4.25
M	2100	0	2100	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	2178	1209 / 0	486 / 0	0 / 0	0 / 0	473 / 0	0 / 0
M	1704	979 / 0	366 / 0	0 / 0	0 / 0	359 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.61 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 33	-84.3	-84.3	0.07 (1)	10.00
B-C	-2365 / 0	-84.3	-84.3	0.16 (1)	5.17
C-D	-4311 / 0	-84.3	-84.3	0.43 (1)	3.79
D-E	-4311 / 0	-84.3	-84.3	0.51 (1)	3.66
E-N	-4311 / 0	-84.3	-84.3	0.51 (1)	3.66
N-F	-4311 / 0	-84.3	-84.3	0.51 (1)	3.66
F-O	-4173 / 0	-84.3	-84.3	0.59 (1)	3.61
O-P	-4173 / 0	-84.3	-84.3	0.59 (1)	3.61
P-Q	-4173 / 0	-84.3	-84.3	0.59 (1)	3.61
Q-G	-4173 / 0	-84.3	-84.3	0.59 (1)	3.61
H-G	-2699 / 0	0.0	0.0	0.60 (1)	5.26
M-B	-2029 / 0	0.0	0.0	0.14 (1)	7.06
M-L	0 / 0	-28.0	-28.0	0.04 (1)	10.00
L-K	0 / 1946	-28.0	-28.0	0.33 (1)	10.00
K-J	0 / 4173	-28.0	-28.0	0.65 (1)	10.00
J-R	0 / 4173	-28.0	-28.0	0.65 (1)	10.00
R-S	0 / 4173	-28.0	-28.0	0.65 (1)	10.00
S-I	0 / 4173	-28.0	-28.0	0.65 (1)	10.00
I-T	0 / 0	-28.0	-28.0	0.31 (1)	10.00
T-U	0 / 0	-28.0	-28.0	0.31 (1)	10.00
U-V	0 / 0	-28.0	-28.0	0.31 (1)	10.00
V-H	0 / 0	-28.0	-28.0	0.31 (1)	10.00

WEBS

MEMB. FR-TO	FORCE (LBS)	MAX CSI (LC)
L-C	-373 / 26	0.05 (1)
C-K	0 / 2678	0.47 (1)
K-D	-571 / 0	0.10 (1)
K-F	0 / 157	0.03 (1)
I-F	-994 / 0	0.17 (1)
I-G	0 / 4736	0.84 (1)
B-L	0 / 2097	0.35 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
N	16-10-12	-101	-101	--	FRONT	VERT	TOTAL
O	18-10-12	-101	-101	--	FRONT	VERT	TOTAL
P	20-10-12	-101	-101	--	FRONT	VERT	TOTAL
Q	22-10-12	-101	-101	--	FRONT	VERT	TOTAL
R	14-11-8	-1340	-1340	--	FRONT	VERT	TOTAL
S	16-10-12	-40	-70	--	FRONT	VERT	TOTAL
T	18-10-12	-40	-70	--	FRONT	VERT	TOTAL
U	20-10-12	-40	-70	--	FRONT	VERT	TOTAL
V	22-10-12	-40	-70	--	FRONT	VERT	TOTAL

DESIGN CRITERIA

[M]

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 063-09
- TPIC 2011

(55 % OF 31.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.27")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 684 (0.43")

CSI: TC=0.60/1.00 (G-H:1) , BC=0.86/1.00 (I-K:1) , WB=0.84/1.00 (G-I:1) , SSI=0.69/1.00 (I-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

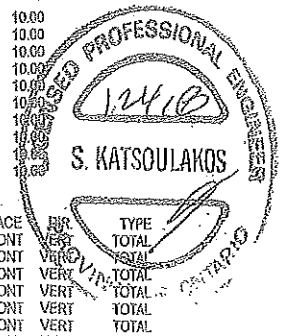
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1658

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.68 (i) (INPUT = 0.90)
JSI METAL= 0.70 (i) (INPUT = 1.00)



DWG NO. TAM5069-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285953	T8Z6	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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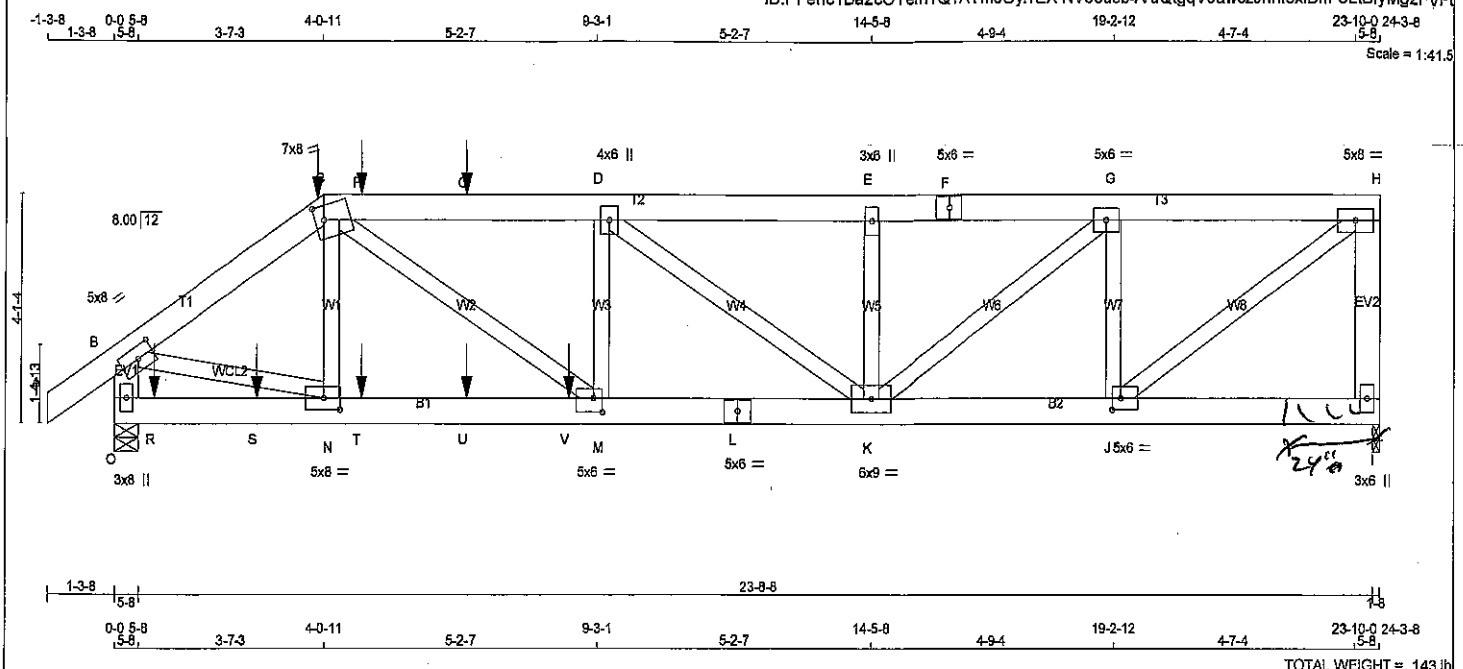

TOTAL WEIGHT = 143 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
I - H	2x6	DRY	No.2	SPF
O - B	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.50	3.75
C	TTWW-m	MT20	7.0	8.0	3.00	2.00
D	TMWW+t	MT20	4.0	6.0		
E	TMW+w	MT20	3.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMWW-t	MT20	5.0	6.0		
H	TMVW-t	MT20	5.0	8.0		
I	BMV1+p	MT20	3.0	6.0		
J	BMWW-t	MT20	5.0	6.0	2.50	2.00
K	BMWWW-t	MT20	6.0	9.0		
L	BS-t	MT20	5.0	6.0		
M	BMWW-t	MT20	5.0	6.0	3.00	2.00
N	BMWW-t	MT20	5.0	8.0	2.50	3.75
O	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 244.5 lbs FACTORED DOWN AT 4-0-11, AND 113.4 lbs FACTORED DOWN AT 4-9-4, AND 101.4 lbs FACTORED DOWN AT 6-9-4 ON TOP CHORD, AND 75.5 lbs FACTORED DOWN AT 9-4, 69.9 lbs FACTORED DOWN AT 2-9-4, 69.9 lbs FACTORED DOWN AT 4-9-4, AND 69.9 lbs FACTORED DOWN AT 6-9-4, AND 1165.0 lbs FACTORED DOWN AT 8-8-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

4c

SCAB JOINT # I & F WITH 2 X 6 #2 SPF 24" LONG 1 SIDE(S) USING 2 ROW(S) 3¼" ~~ARROW SPIRAL~~/COMMON WIRE NAILS AT 2" O/C STAGGERED (6 NAILS/SCAB). REQUIRED FOR 1½" BRG PROVISION.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	1897	0	1897	0	0	1-8	2-8
O	2736	0	2736	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1550	873 / 0	345 / 0	0 / 0	0 / 0	333 / 0	0 / 0
O	2228	1267 / 0	487 / 0	0 / 0	0 / 0	474 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.73 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 33	-84.3	-84.3	0.07 (1)	10.00	
B- C	-3252 / 0	-84.3	-84.3	0.19 (1)	4.52	
C- P	-4470 / 0	-84.3	-84.3	0.41 (1)	3.73	
P- Q	-4470 / 0	-84.3	-84.3	0.41 (1)	3.73	
Q- D	-4470 / 0	-84.3	-84.3	0.41 (1)	3.73	
D- E	-3636 / 0	-84.3	-84.3	0.26 (1)	4.26	
E- F	-3636 / 0	-84.3	-84.3	0.22 (1)	4.30	
F- G	-3636 / 0	-84.3	-84.3	0.22 (1)	4.30	
G- H	-2304 / 0	-84.3	-84.3	0.18 (1)	5.21	
I- H	-1836 / 0	0.0	0.0	0.27 (1)	7.34	
O- B	-2681 / 0	0.0	0.0	0.19 (1)	6.32	
O- R	0 / 0	-28.0	-28.0	0.19 (2)	10.00	
R- S	0 / 0	-28.0	-28.0	0.19 (2)	10.00	
S- N	0 / 0	-28.0	-28.0	0.19 (2)	10.00	
N- T	0 / 2692	-28.0	-28.0	0.67 (1)	10.00	
T- U	0 / 2692	-28.0	-28.0	0.67 (1)	10.00	
U- V	0 / 2692	-28.0	-28.0	0.67 (1)	10.00	
V- M	0 / 2692	-28.0	-28.0	0.67 (1)	10.00	
M- L	0 / 4470	-28.0	-28.0	0.77 (1)	10.00	
L- K	0 / 4470	-28.0	-28.0	0.77 (1)	10.00	
K- J	0 / 2304	-28.0	-28.0	0.37 (1)	10.00	
J- I	0 / 0	-28.0	-28.0	0.08 (3)	10.00	

WEBS FR-TO	MAX. FACTORED MEMB. FORCE (LBS)	MAX CSI (LC)
N- C	-170 / 231	0.04 (3)
C- M	0 / 2157	0.38 (1)
M- D	0 / 238	0.04 (3)
D- K	-1023 / 0	0.49 (1)
K- E	-375 / 0	0.06 (1)
K- G	0 / 1678	0.30 (1)
J- G	-1485 / 0	0.25 (1)
J- H	0 / 2636	0.50 (1)
B- N	0 / 2759	0.49 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	4-0-11	-23	-25	---	FRONT	VERT	DEAD
C	4-0-11	-222	-222	---	FRONT	VERT	SNOW
P	4-9-4	-113	-113	---	BACK	VERT	TOTAL
Q	6-9-4	-101	-101	---	BACK	VERT	TOTAL
R	9-4	-43	-76	---	BACK	VERT	TOTAL
S	2-9-4	-40	-70	---	BACK	VERT	TOTAL
T	4-9-4	-40	-70	---	BACK	VERT	TOTAL
U	6-9-4	-40	-70	---	BACK	VERT	TOTAL
V	8-8-8	-1165	-1165	---	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER 5228 S KATSOULAKOS PROVINCE OF ONTARIO

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.41/1.00 (C-D:1) , BC=0.77/1.00 (K-M:1) , WB=0.50/1.00 (H-J:1) , SSI=0.75/1.00 (M-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.66 (L) (INPUT = 1.00)

DWG NO. TAM2-6884-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285994	T9	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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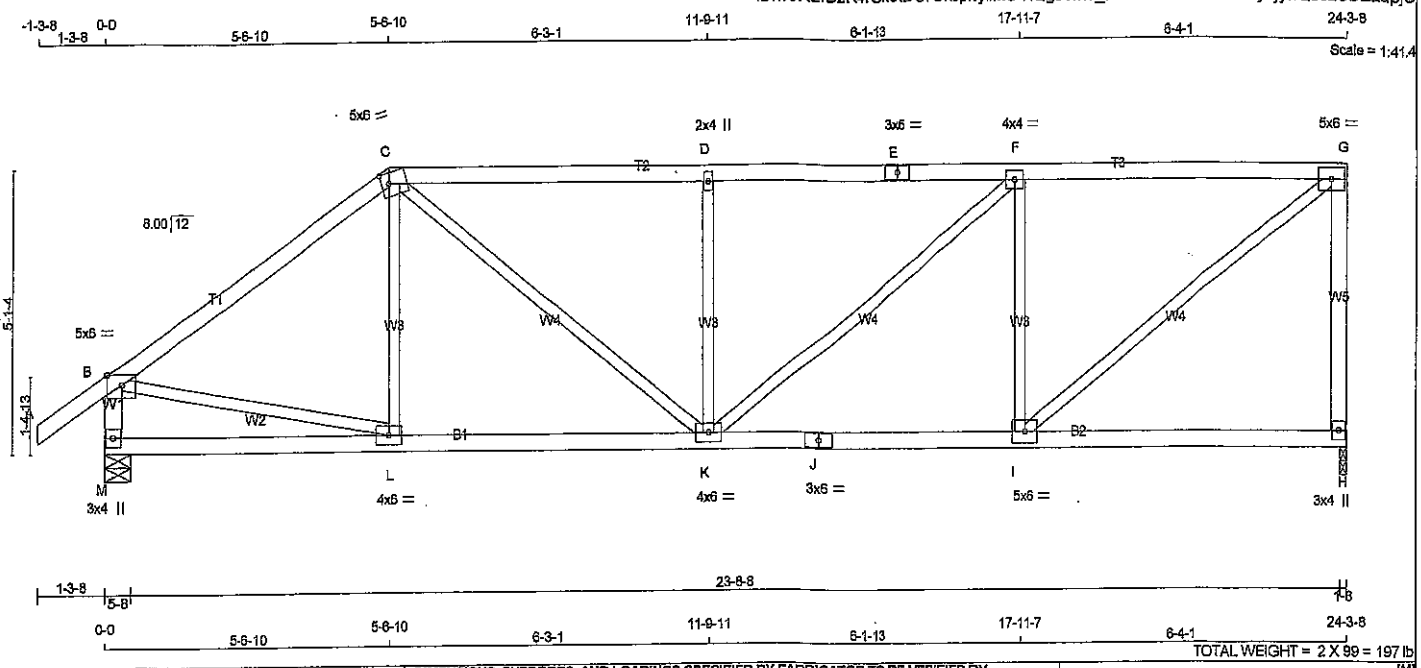

TOTAL WEIGHT = 2 X 99 = 197 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESGR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW-p	MT20	5.0	8.0	Edge	
C	TTWW-m	MT20	5.0	6.0	2.25	1.75
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H	1363	0	1363	0	0	1-8	1-8
M	1479	0	1479	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1119	821 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
M	1198	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.34 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	L- C	-49 / 204	0.05 (3)
B- C	-1493 / 0	-84.3	-84.3	0.55 (1)	4.69	B- L	0 / 1263	0.28 (1)
C- D	-1797 / 0	-84.3	-84.3	0.55 (1)	4.37	I- G	0 / 1776	0.40 (1)
D- E	-1797 / 0	-84.3	-84.3	0.56 (1)	4.34	C- K	0 / 702	0.16 (1)
E- F	-1797 / 0	-84.3	-84.3	0.56 (1)	4.34	I- F	-877 / 0	0.34 (1)
F- G	-1416 / 0	-84.3	-84.3	0.53 (1)	4.81	K- D	-566 / 0	0.22 (1)
H- G	-1282 / 0	0.0	0.0	0.57 (1)	7.10	K- F	0 / 484	0.11 (1)
M- B	-1417 / 0	0.0	0.0	0.15 (1)	6.85			
M- L	0 / 0	-28.0	-28.0	0.24 (3)	10.00			
L- K	0 / 1240	-28.0	-28.0	0.39 (2)	10.00			
K- J	0 / 1416	-28.0	-28.0	0.44 (2)	10.00			
J- I	0 / 1416	-28.0	-28.0	0.44 (2)	10.00			
I- H	0 / 0	-28.0	-28.0	0.27 (3)	10.00			

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 5.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.57/1.00 (G-H:1) , BC=0.44/1.00 (I-K:2) , WB=0.40/1.00 (G-I:1) , SSI=0.25/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (K) (INPUT = 0.90)
JSI METAL= 0.44 (J) (INPUT = 1.00)

DWG NO. TAM 3480 -18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285994	T10	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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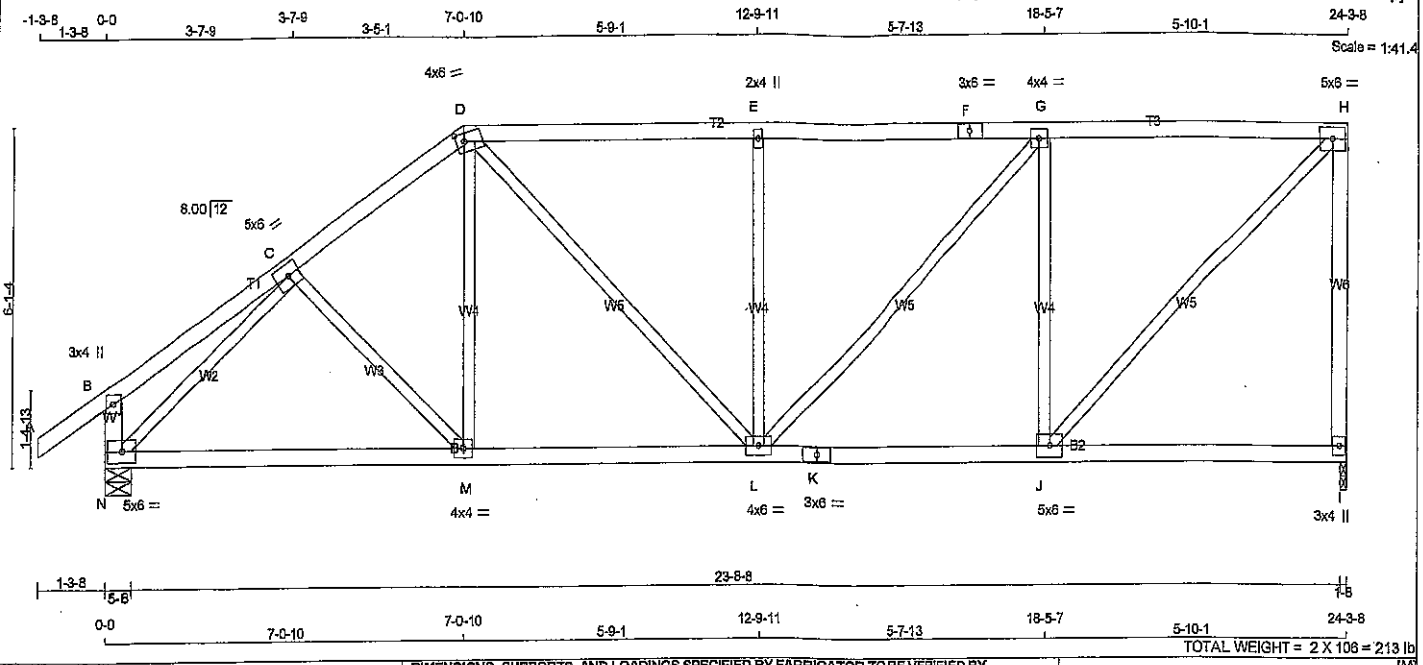

TOTAL WEIGHT = 2 X 106 = 213 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW-m	MT20	4.0	6.0	1.75	1.75
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TMVW-t	MT20	5.0	6.0		
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	5.0	6.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	4.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	1363	0	1363	0	0	1-8	1-8
N	1479	0	1479	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1119	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
N	1198	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.86 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	
B-C	0 / 17	-84.3	-84.3	0.15 (1)	10.00	
C-D	-1460 / 0	-84.3	-84.3	0.18 (1)	5.22	
D-E	-1473 / 0	-84.3	-84.3	0.43 (1)	4.86	
E-F	-1473 / 0	-84.3	-84.3	0.44 (1)	4.86	
F-G	-1473 / 0	-84.3	-84.3	0.44 (1)	4.86	
G-H	-1103 / 0	-84.3	-84.3	0.42 (1)	5.46	
I-H	-1298 / 0	0.0	0.0	0.94 (1)	7.08	
N-B	-233 / 0	0.0	0.0	0.02 (1)	7.81	
N-M	0 / 1196	-28.0	-28.0	0.45 (2)	10.00	
M-L	0 / 1202	-28.0	-28.0	0.47 (2)	10.00	
L-K	0 / 1103	-28.0	-28.0	0.37 (2)	10.00	
K-J	0 / 1103	-28.0	-28.0	0.37 (2)	10.00	
J-I	0 / 0	-28.0	-28.0	0.24 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-M	0 / 118	0.03 (3)
M-D	0 / 307	0.07 (2)
N-C	-1689 / 0	0.64 (1)
J-H	0 / 1557	0.35 (1)
D-L	0 / 385	0.09 (1)
J-G	-912 / 0	0.53 (1)
L-E	-520 / 0	0.30 (1)
L-G	0 / 590	0.12 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.94/1.00 (H-I:1) , BC=0.47/1.00 (L-M:2) , WB=0.64/1.00 (C-N:1) , SSI=0.23/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (D) (INPUT = 0.90)
JSI METAL= 0.41 (C) (INPUT = 1.00)

DWG NO. TAM 3909 -18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285994	T11	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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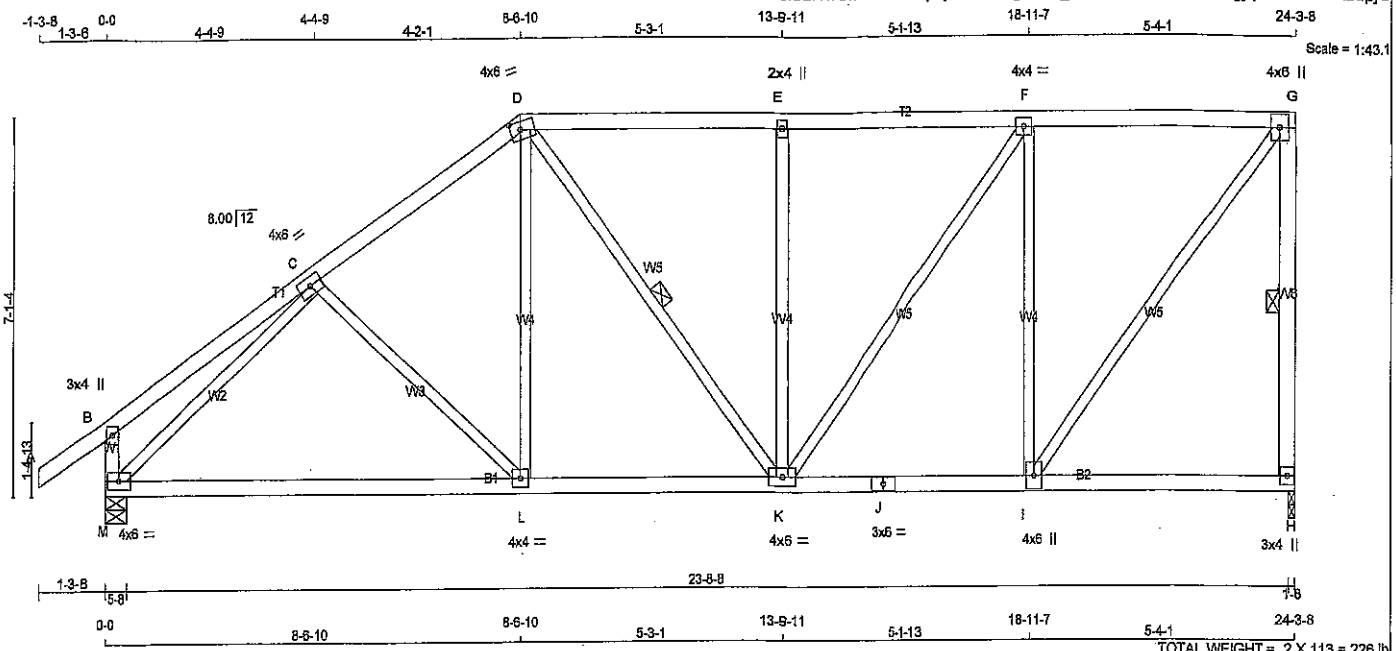

TOTAL WEIGHT = 2 X 113 = 226 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW-m	MT20	4.0	6.0	1.75	2.50
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW+p	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWW+t	MT20	4.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	1363	0	1363	0	0	1-8	1-8
M	1479	0	1479	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1119	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
M	1198	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-H, D-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	C- L	-117 / 85	0.07 (1)	
B- C	0 / 23	-84.3	-84.3	0.24 (1)	10.00	L- D	0 / 414	0.09 (2)	
C- D	-1390 / 0	-84.3	-84.3	0.28 (1)	5.20	M- C	-1661 / 0	0.93 (1)	
D- E	-1226 / 0	-84.3	-84.3	0.34 (1)	5.37	I- G	0 / 1426	0.32 (1)	
E- F	-1227 / 0	-84.3	-84.3	0.35 (1)	5.35	D- K	-6 / 141	0.03 (1)	
F- G	-879 / 0	-84.3	-84.3	0.34 (1)	6.07	I- F	-946 / 0	0.63 (1)	
H- G	-1305 / 0	0.0	0.0	0.30 (1)	5.66	K- E	-474 / 0	0.41 (1)	
M- B	-255 / 0	0.0	0.0	0.03 (1)	7.81	K- F	0 / 576	0.13 (1)	
M- L	0 / 1223	-28.0	-28.0	0.59 (2)	10.00				
L- K	0 / 1141	-28.0	-28.0	0.59 (2)	10.00				
K- J	0 / 879	-28.0	-28.0	0.31 (2)	10.00				
J- I	0 / 879	-28.0	-25.0	0.31 (2)	10.00				
I- H	0 / 0	-28.0	-28.0	0.21 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.23")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 779 (0.37")

CSI: TC=0.35/1.00 (E-F:1) , BC=0.59/1.00 (K-L:2) , WB=0.93/1.00 (C-M:1) , SSI=0.21/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BENO=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (M) (INPUT = 0.90)
JSI METAL= 0.40 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
1676
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 3990 -18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285994	T12	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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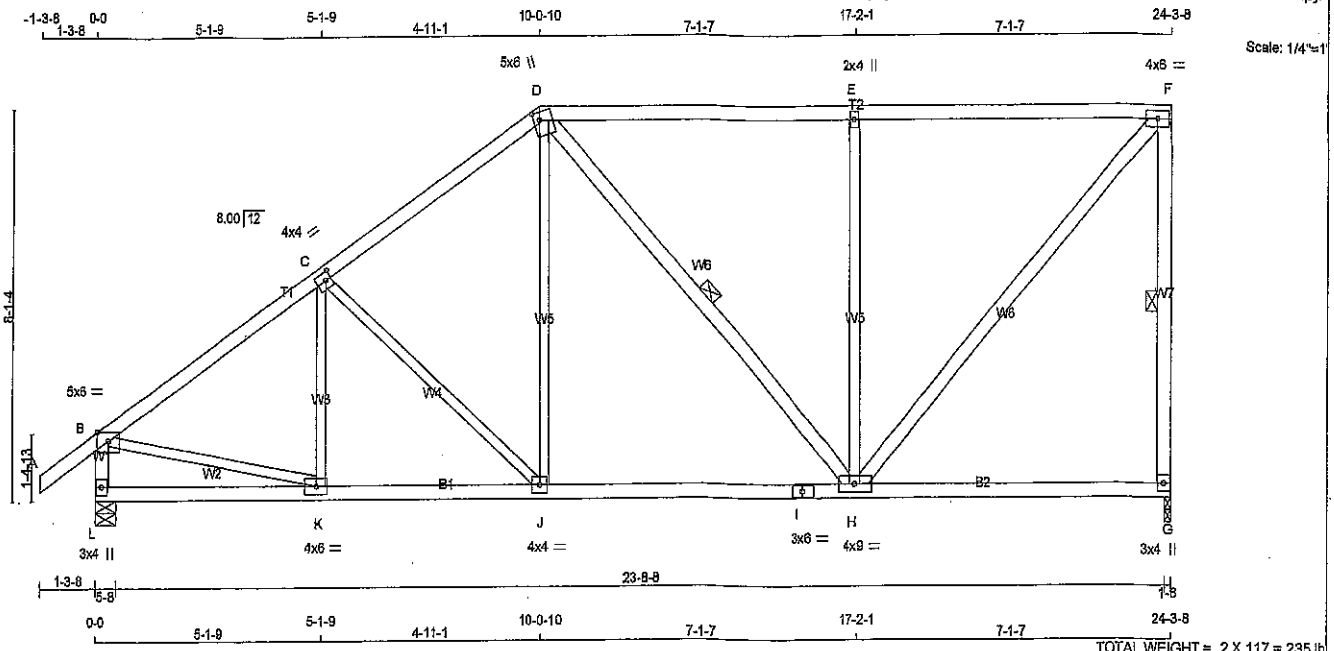

TOTAL WEIGHT = 2 X 117 = 235 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1363	0	1363	0	0	1-8	1-8
L	1479	0	1479	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1119	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
L	1198	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.81 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00
B- C	-1529 / 0	-84.3	-84.3	0.43 (1)	4.83
C- D	-1275 / 0	-84.3	-84.3	0.40 (1)	5.20
D- E	-967 / 0	-84.3	-84.3	0.80 (1)	4.81
E- F	-967 / 0	-84.3	-84.3	0.81 (1)	4.81
G- F	-1264 / 0	0.0	0.0	0.38 (1)	5.69
L- B	-1416 / 0	0.0	0.0	0.15 (1)	6.85
L- K	0 / 0	-28.0	-28.0	0.16 (3)	10.00
K- J	0 / 1285	-28.0	-28.0	0.33 (2)	10.00
J- I	0 / 1041	-28.0	-28.0	0.49 (2)	10.00
I- H	0 / 1041	-28.0	-28.0	0.49 (2)	10.00
H- G	0 / 0	-28.0	-28.0	0.37 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-123 / 133	0.04 (1)
C- J	-347 / 0	0.29 (1)
J- D	0 / 436	0.10 (2)
D- H	-110 / 0	0.06 (1)
H- E	-745 / 0	0.94 (1)
H- F	0 / 1432	0.23 (1)
B- K	0 / 1323	0.30 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.81/1.00 (E-F:1) , BC=0.49/1.00 (H-J:2) , WB=0.94/1.00 (E-H:1) , SSI=0.29/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	822	2284	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.59 (K) (INPUT = 0.90)
JSI METAL= 0.36 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 3991-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43994	DRWG NO.
286039	T13	5	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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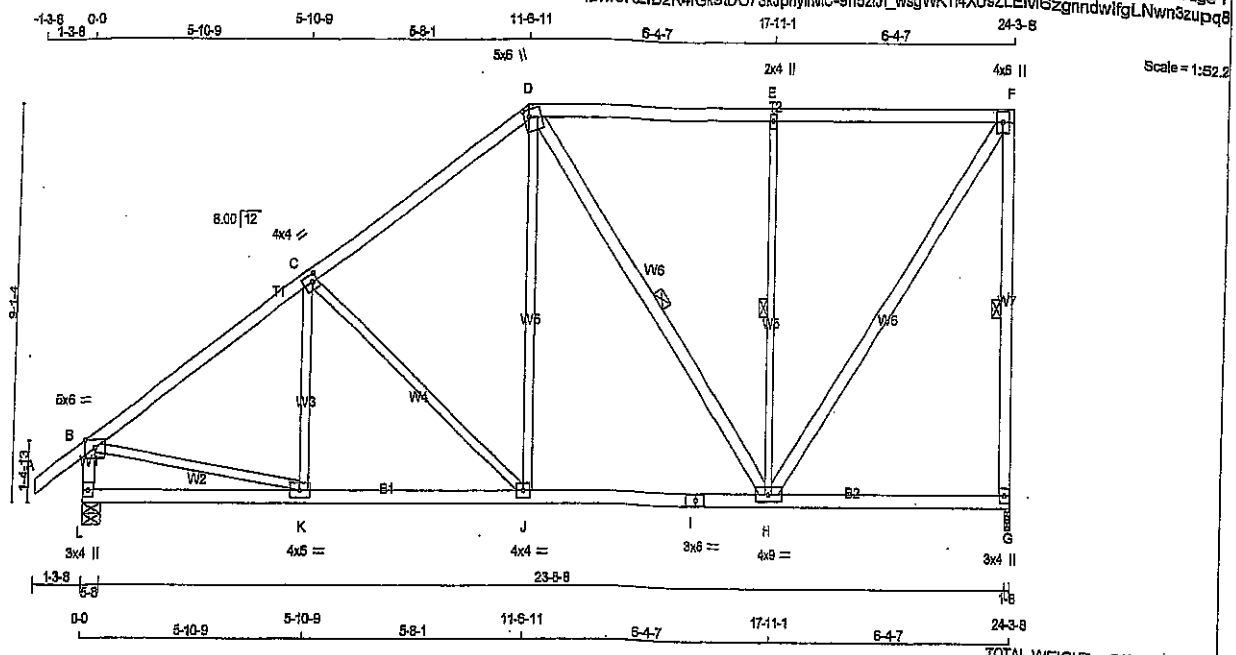


TOTAL WEIGHT = 5 X 123 = 614 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
O	TTVW+m	MT20	5.0	6.0	2.00	1.50
E	TMV+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1383	0	1383	0	0	1-8	1-8
L	1479	0	1479	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1119	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
L	1198	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.61 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

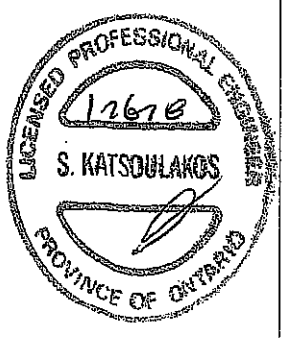
1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.3	-84.3	0.11 (1)		10.00
B-C	-1531 / 0	-84.3	-84.3	0.59 (1)		4.61
C-D	-1167 / 0	-84.3	-84.3	0.54 (1)		5.16
D-E	-790 / 0	-84.3	-84.3	0.61 (1)		5.73
E-F	-790 / 0	-84.3	-84.3	0.62 (1)		5.73
G-F	-1292 / 0	0.0	0.0	0.50 (1)		5.68
L-B	-1412 / 0	0.0	0.0	0.15 (1)		6.86
L-K	0 / 0	-28.0	-28.0	0.23 (3)		10.00
K-J	0 / 1300	-28.0	-28.0	0.38 (2)		10.00
J-I	0 / 948	-28.0	-28.0	0.40 (2)		10.00
I-H	0 / 948	-28.0	-28.0	0.40 (2)		10.00
H-G	0 / 0	-28.0	-28.0	0.29 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K-C	-58 / 203	0.05 (3)
C-J	-474 / 0	0.56 (1)
J-D	0 / 490	0.11 (2)
D-H	-270 / 0	0.17 (1)
H-E	-666 / 0	0.35 (1)
H-F	0 / 1344	0.22 (1)
B-K	0 / 1322	0.30 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.62/1.00 (E-F:1) , BC=0.40/1.00 (H-J:2) , WB=0.56/1.00 (C-J:1) , SSI=0.26/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2264	1658

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.32 (K) (INPUT = 1.00)

DWG NO. TAM 3993 -12
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285953	T14	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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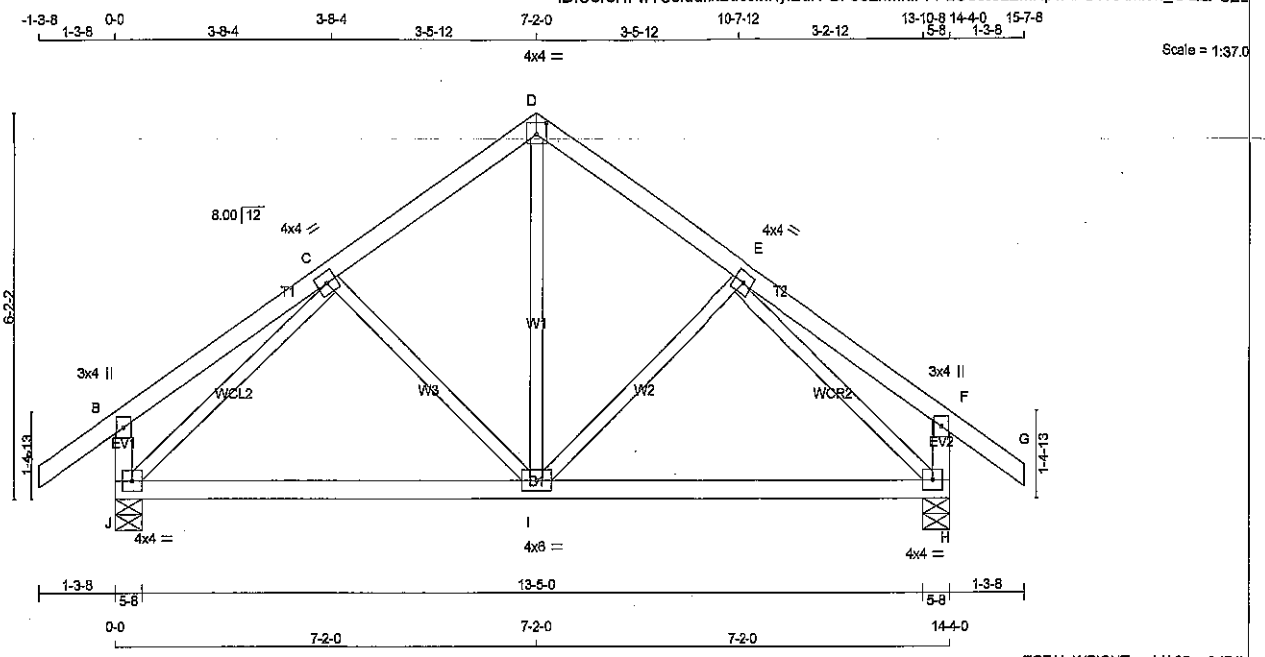

TOTAL WEIGHT = 4 X 62 = 247 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
J	920	0	920	0	0	5-8	5-8
H	920	0	920	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	739	437 / 0	151 / 0	0 / 0	0 / 0	152 / 0	0 / 0
H	739	437 / 0	151 / 0	0 / 0	0 / 0	152 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	I- D	0 / 460	0.10 (1)
B- C	0 / 19	-84.3	-84.3	0.17 (1)	10.00	I- E	-152 / 44	0.06 (1)
C- D	-645 / 0	-84.3	-84.3	0.13 (1)	6.25	C- I	-152 / 44	0.06 (1)
D- E	-645 / 0	-84.3	-84.3	0.13 (1)	6.25	J- C	-879 / 0	0.35 (1)
E- F	0 / 19	-84.3	-84.3	0.17 (1)	10.00	E- H	-879 / 0	0.35 (1)
F- G	0 / 32	-84.3	-84.3	0.11 (1)	10.00			
J- B	-232 / 0	0.0	0.0	0.02 (1)	7.81			
H- F	-232 / 0	0.0	0.0	0.02 (1)	7.81			
J- I	0 / 631	-28.0	-28.0	0.49 (2)	10.00			
I- H	0 / 631	-28.0	-28.0	0.49 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.48")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.48")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.17/1.00 (E-F:1) , BC=0.49/1.00 (H-I:2) , WB=0.35/1.00 (E-H:1) , SSI=0.17/1.00 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (D) (INPUT = 0.90)
JSI METAL= 0.31 (C) (INPUT = 1.00)

DWG NO. TAM2/222-T14
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285952	T15	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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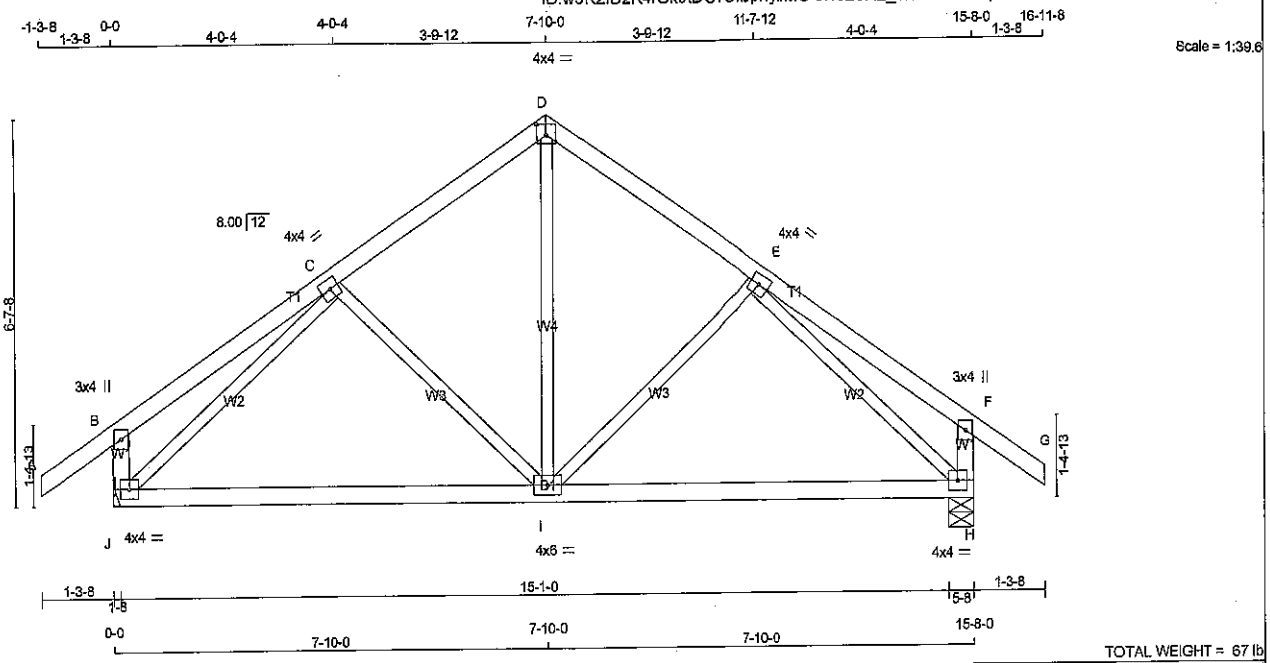

TOTAL WEIGHT = 67 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	995	0	995	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
H	995	0	995	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	800	471 / 0	165 / 0	0 / 0	0 / 0	165 / 0	0 / 0
H	800	471 / 0	165 / 0	0 / 0	0 / 0	165 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	
B- C	0 / 21	-84.3	-84.3	0.20 (1)	10.00	
C- D	-717 / 0	-84.3	-84.3	0.16 (1)	6.25	
D- E	-717 / 0	-84.3	-84.3	0.16 (1)	6.25	
E- F	0 / 21	-84.3	-84.3	0.20 (1)	10.00	
F- G	0 / 32	-84.3	-84.3	0.11 (1)	10.00	
J- B	-243 / 0	0.0	0.0	0.02 (1)	7.81	
H- F	-243 / 0	0.0	0.0	0.02 (1)	7.81	
J- I	0 / 709	-28.0	-28.0	0.58 (2)	10.00	
I- H	0 / 709	-28.0	-28.0	0.58 (2)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- D	0 / 516	0.12 (1)
I- E	-179 / 43	0.09 (1)
C- I	-179 / 43	0.09 (1)
J- C	-975 / 0	0.46 (1)
E- H	-975 / 0	0.46 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.52")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.52")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.20 (8-C:1) , BC=0.58 (H-I:2) , WB=0.46 (C-J:1) , SSI=0.19 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

ISI GRIP= 0.87 (D) (INPUT = 0.90)
ISI METAL= 0.35 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
9/11/17
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM45576-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285952	G15	1	1	TRUSS DESC.	

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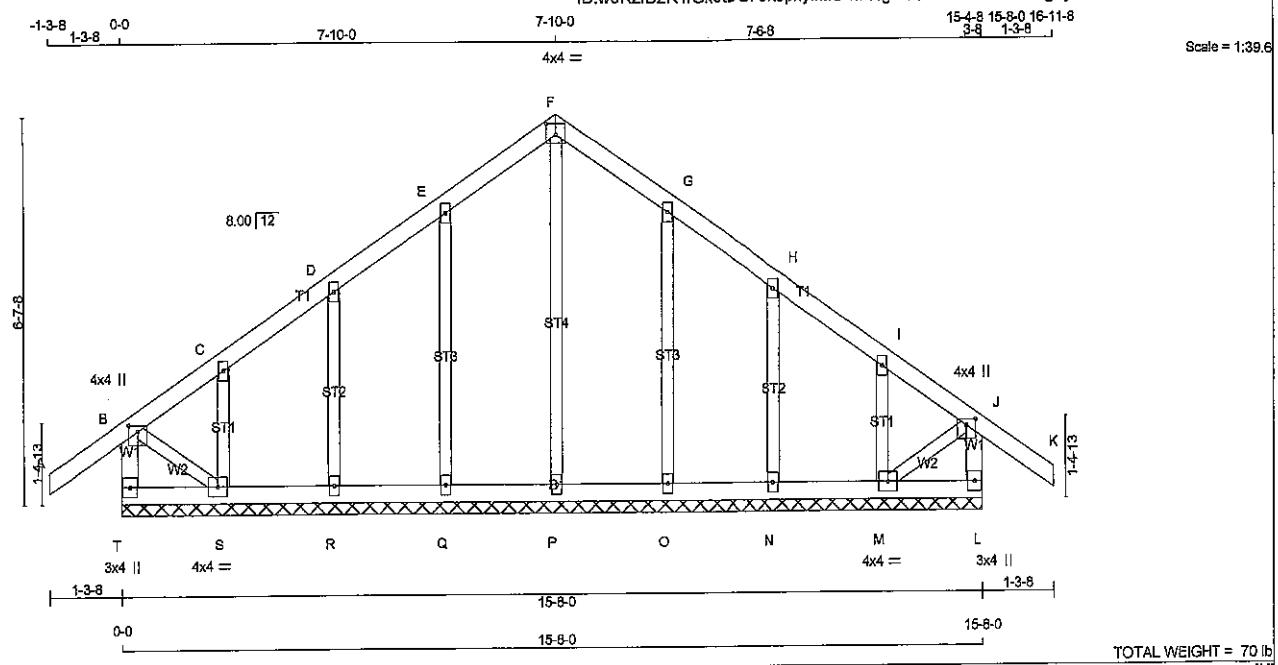

TOTAL WEIGHT = 70 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
T - B	2x4	DRY	No.2	SPF
A - F	2x4	DRY	No.2	SPF
F - K	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C, D, E, G, H, I						
C	TMW+w	MT20	2.0	4.0		
F	TTW-p	MT20	4.0	4.0	2.25	2.00
J	TMVW+p	MT20	4.0	4.0	1.25	2.00
L	BMV1+p	MT20	3.0	4.0		
M	BMWW1-t	MT20	4.0	4.0		
N, O, P, Q, R						
N	BMW1+w	MT20	2.0	4.0		
S	BMWW1-t	MT20	4.0	4.0		
T	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T-B	-253 / 0	0.0	0.0	0.03 (1)	7.81	P-F	-129 / 0	0.09 (1)
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	Q-E	-189 / 0	0.08 (1)
B-C	-46 / 0	-84.3	-84.3	0.11 (1)	6.25	R-D	-172 / 0	0.04 (1)
C-D	-15 / 0	-84.3	-84.3	0.04 (1)	6.25	S-C	-108 / 0	0.02 (1)
D-E	-14 / 0	-84.3	-84.3	0.05 (1)	6.25	O-G	-189 / 0	0.08 (1)
E-F	-21 / 0	-84.3	-84.3	0.05 (1)	6.25	N-H	-172 / 0	0.04 (1)
F-G	-21 / 0	-84.3	-84.3	0.05 (1)	6.25	M-I	-108 / 0	0.02 (1)
G-H	-14 / 0	-84.3	-84.3	0.05 (1)	6.25	B-S	0 / 23	0.01 (1)
H-I	-15 / 0	-84.3	-84.3	0.04 (1)	6.25	M-J	0 / 23	0.01 (1)
I-J	-46 / 0	-84.3	-84.3	0.11 (1)	6.25			
J-K	0 / 32	-84.3	-84.3	0.11 (1)	10.00			
L-J	-253 / 0	0.0	0.0	0.03 (1)	7.81			
T-S	0 / 0	-28.0	-28.0	0.03 (2)	10.00			
S-R	0 / 16	-28.0	-28.0	0.03 (2)	10.00			
R-Q	0 / 12	-28.0	-28.0	0.02 (2)	10.00			
Q-P	0 / 9	-28.0	-28.0	0.02 (2)	10.00			
P-O	0 / 9	-28.0	-28.0	0.02 (2)	10.00			
O-N	0 / 12	-28.0	-28.0	0.02 (2)	10.00			
N-M	0 / 16	-28.0	-28.0	0.03 (2)	10.00			
M-L	0 / 0	-28.0	-28.0	0.03 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11 (J-K:1) , BC=0.03 (R-S:2) , WB=0.09 (F-P:1) , SSI=0.08 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (B) (INPUT = 0.90)
JSI METAL= 0.05 (G) (INPUT = 1.00)

DWG NO. TAM45613 17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285952	T15A	3	1	TRUSS DESC.	

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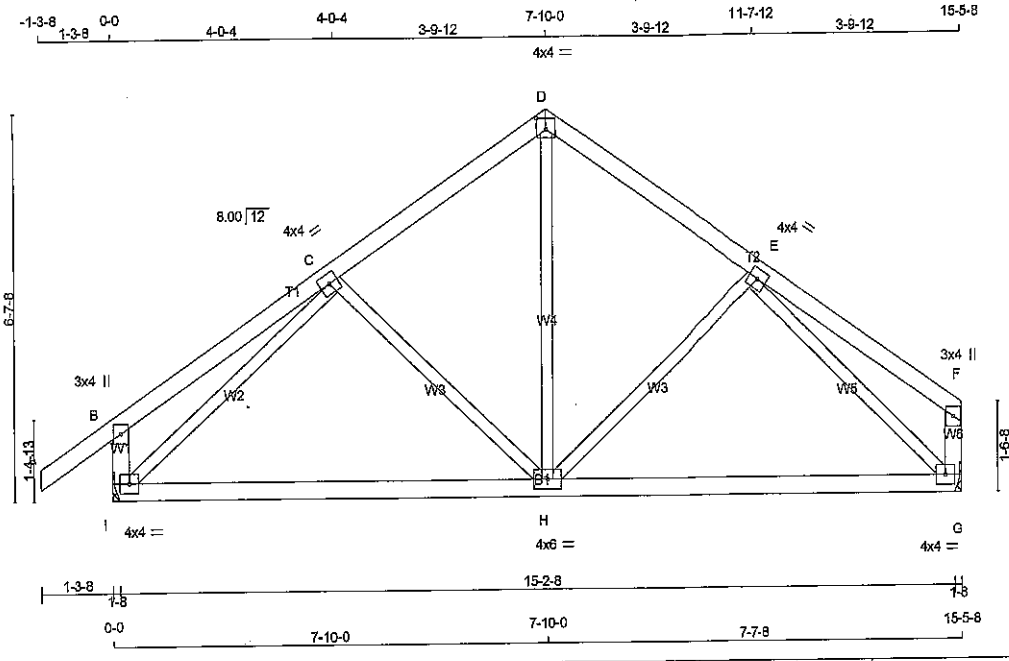

TOTAL WEIGHT = 3 X 65 = 194 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BMWWW-t	MT20	4.0	6.0		
I	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	983	0	983	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
G	868	0	868	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	791	466 / 0	162 / 0	0 / 0	0 / 0	163 / 0	0 / 0
G	712	395 / 0	162 / 0	0 / 0	0 / 0	155 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	
B- C	0 / 21	-84.3	-84.3	0.20 (1)	10.00	
C- D	-698 / 0	-84.3	-84.3	0.15 (1)	6.25	
D- E	-697 / 0	-84.3	-84.3	0.15 (1)	6.25	
E- F	0 / 21	-84.3	-84.3	0.19 (1)	10.00	
I- B	-242 / 0	0.0	0.0	0.02 (1)	7.81	
G- F	-118 / 0	0.0	0.0	0.01 (1)	7.81	
I- H	0 / 696	-28.0	-28.0	0.56 (2)	10.00	
H- G	0 / 672	-28.0	-28.0	0.56 (2)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- D	0 / 494	0.11 (1)
H- E	-146 / 53	0.07 (1)
C- H	-182 / 41	0.09 (1)
I- C	-957 / 0	0.45 (1)
E- G	-948 / 0	0.42 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.52")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.52")
CALCULATED VERT. DEFL.(TL) = L/ 981 (0.19")

CSI: TC=0.20 (B-C:1) , BC=0.56 (H-I:2) , WB=0.45 (C-I:1) , SSI=0.19 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (D) (INPUT = 0.90)
JSI METAL= 0.34 (C) (INPUT = 1.00)

DWG NO. TAM45577-17

STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43854	DRWG NO.
285952	G15A	1	1	TRUSS DESC.	

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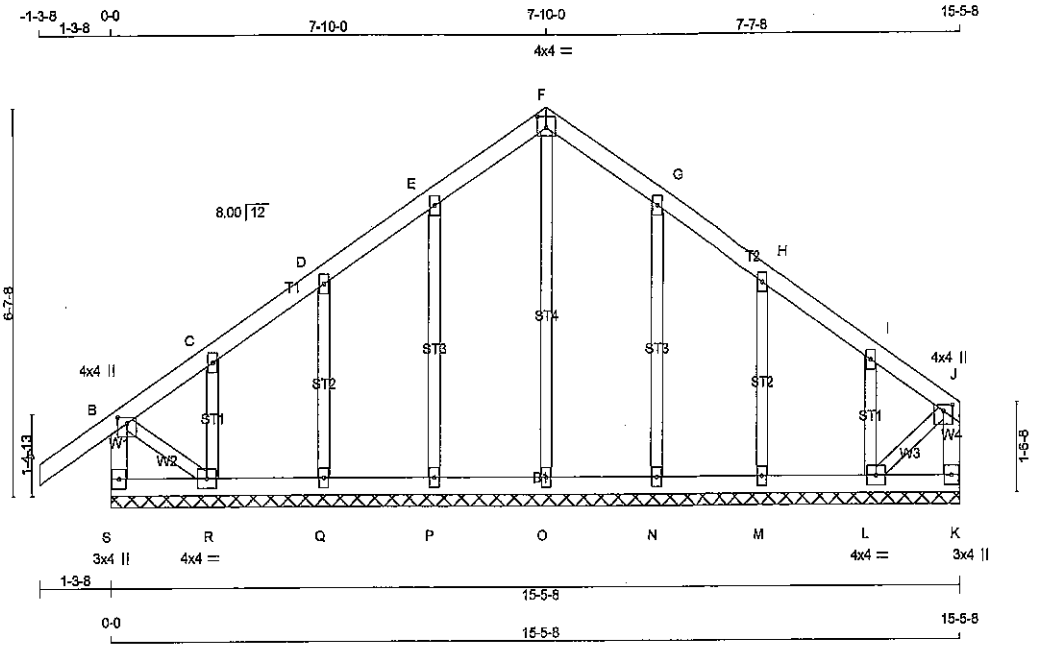

TOTAL WEIGHT = 68 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
S - B	2x4	DRY	No.2	SPF
A - F	2x4	DRY	No.2	SPF
F - J	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
S - K	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C, D, E, G, H, I						
C	TMW+w	MT20	2.0	4.0		
F	TTW-p	MT20	4.0	4.0	2.25	2.00
J	TMVW+p	MT20	4.0	4.0	1.25	2.00
K	BMV1+p	MT20	3.0	4.0		
L	BMWW1-t	MT20	4.0	4.0		
M, N, O, P, Q						
M	BMV1+w	MT20	2.0	4.0		
R	BMWW1-t	MT20	4.0	4.0		
S	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
FR-TO					
S-B	-258 / 0	0.0	0.0	0.03 (1)	7.81
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00
B-C	-50 / 0	-84.3	-84.3	0.11 (1)	6.25
C-D	-19 / 0	-84.3	-84.3	0.04 (1)	6.25
D-E	-18 / 0	-84.3	-84.3	0.05 (1)	6.25
E-F	-25 / 0	-84.3	-84.3	0.05 (1)	6.25
F-G	-25 / 0	-84.3	-84.3	0.05 (1)	6.25
G-H	-17 / 0	-84.3	-84.3	0.05 (1)	6.25
H-I	-24 / 0	-84.3	-84.3	0.04 (1)	6.25
I-J	-21 / 0	-84.3	-84.3	0.04 (1)	6.25
K-J	-84 / 0	0.0	0.0	0.01 (1)	7.81
S-R	0 / 0	-28.0	-28.0	0.03 (2)	10.00
R-Q	0 / 20	-28.0	-28.0	0.03 (2)	10.00
Q-P	0 / 16	-28.0	-28.0	0.02 (2)	10.00
P-O	0 / 13	-28.0	-28.0	0.02 (2)	10.00
O-N	0 / 13	-28.0	-28.0	0.02 (2)	10.00
N-M	0 / 16	-28.0	-28.0	0.02 (2)	10.00
M-L	0 / 20	-28.0	-28.0	0.02 (2)	10.00
L-K	0 / 0	-28.0	-28.0	0.02 (2)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
O-F	-124 / 0	0.09 (1)
P-E	-169 / 0	0.08 (1)
Q-D	-172 / 0	0.04 (1)
R-C	-108 / 0	0.02 (1)
N-G	-191 / 0	0.08 (1)
M-H	-162 / 0	0.04 (1)
L-I	-162 / 0	0.03 (1)
B-R	0 / 28	0.01 (1)
L-J	0 / 32	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11 (A-B:1) , BC=0.03 (Q-R:2) , WB=0.09 (F-O:1) , SSI=0.08 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (B) (INPUT = 0.90)
JSI METAL= 0.05 (G) (INPUT = 1.00)

DWG NO. TAM 45614 17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285952	T24	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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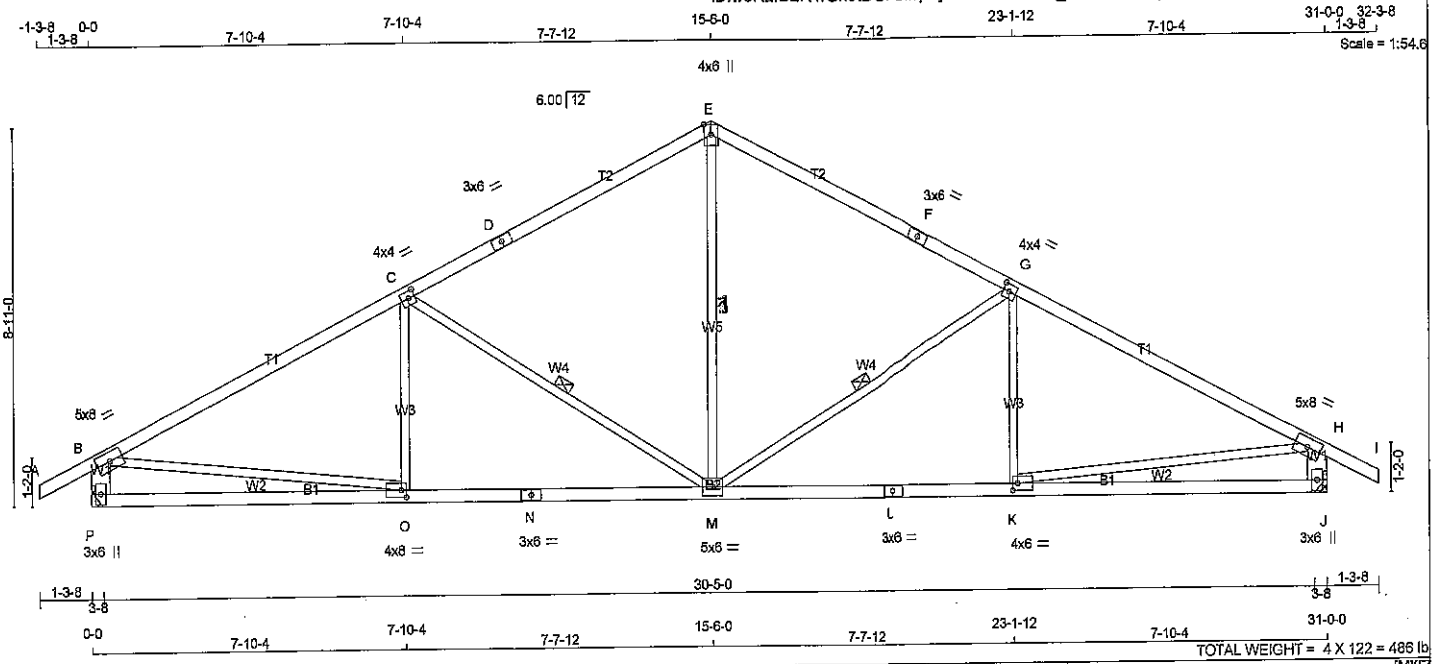

TOTAL WEIGHT = 4 X 122 = 486 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
P - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	6.0		
E	TTW+p	MT20	4.0	6.0	Edge	
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
H	TMVW-t	MT20	5.0	8.0		
J	BMV1+p	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	6.0	2.00	1.50
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	5.0	8.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	6.0	2.00	1.50
P	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
P	1854	0	1854	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	
J	1854	0	1854	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1506	863 / 0	326 / 0	0 / 0	0 / 0	316 / 0	0 / 0
J	1506	863 / 0	326 / 0	0 / 0	0 / 0	316 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.27 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-M, C-M. EMP

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FR-TO	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	FR-TO	MAX CSI (LC)
A-B	0 / 26		-84.3	-84.3	0.11 (1)	10.00	M-E	0 / 1098		0.25 (1)
B-C	-2529 / 0		-84.3	-84.3	0.94 (1)	3.27	M-G	-840 / 0		0.48 (1)
C-D	-1803 / 0		-84.3	-84.3	0.76 (1)	3.96	K-G	-9 / 307		0.07 (3)
D-E	-1803 / 0		-84.3	-84.3	0.76 (1)	3.96	C-M	-840 / 0		0.48 (1)
E-F	-1803 / 0		-84.3	-84.3	0.76 (1)	3.96	O-C	-9 / 307		0.07 (3)
F-G	-1803 / 0		-84.3	-84.3	0.76 (1)	3.96	B-O	0 / 2306		0.52 (1)
G-H	-2529 / 0		-84.3	-84.3	0.94 (1)	3.27	K-H	0 / 2306		0.52 (1)
H-I	0 / 26		-84.3	-84.3	0.11 (1)	10.00				
P-B	-1764 / 0		0.0	0.0	0.11 (1)	7.48				
J-H	-1764 / 0		0.0	0.0	0.11 (1)	7.48				
P-O	0 / 0		-28.0	-28.0	0.42 (3)	10.00				
O-N	0 / 2293		-28.0	-28.0	0.68 (2)	10.00				
N-M	0 / 2293		-28.0	-28.0	0.68 (2)	10.00				
M-L	0 / 2293		-28.0	-28.0	0.68 (2)	10.00				
L-K	0 / 2293		-28.0	-28.0	0.68 (2)	10.00				
K-J	0 / 0		-28.0	-28.0	0.42 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.30")

CSI: TC=0.94 (G-H:1) , BC=0.68 (K-M:2) , WB=0.52 (H-K:1) , SSI=0.29 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (K) (INPUT = 0.90)
JSI METAL= 0.69 (L) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
9467
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 45578-17
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285864	T30	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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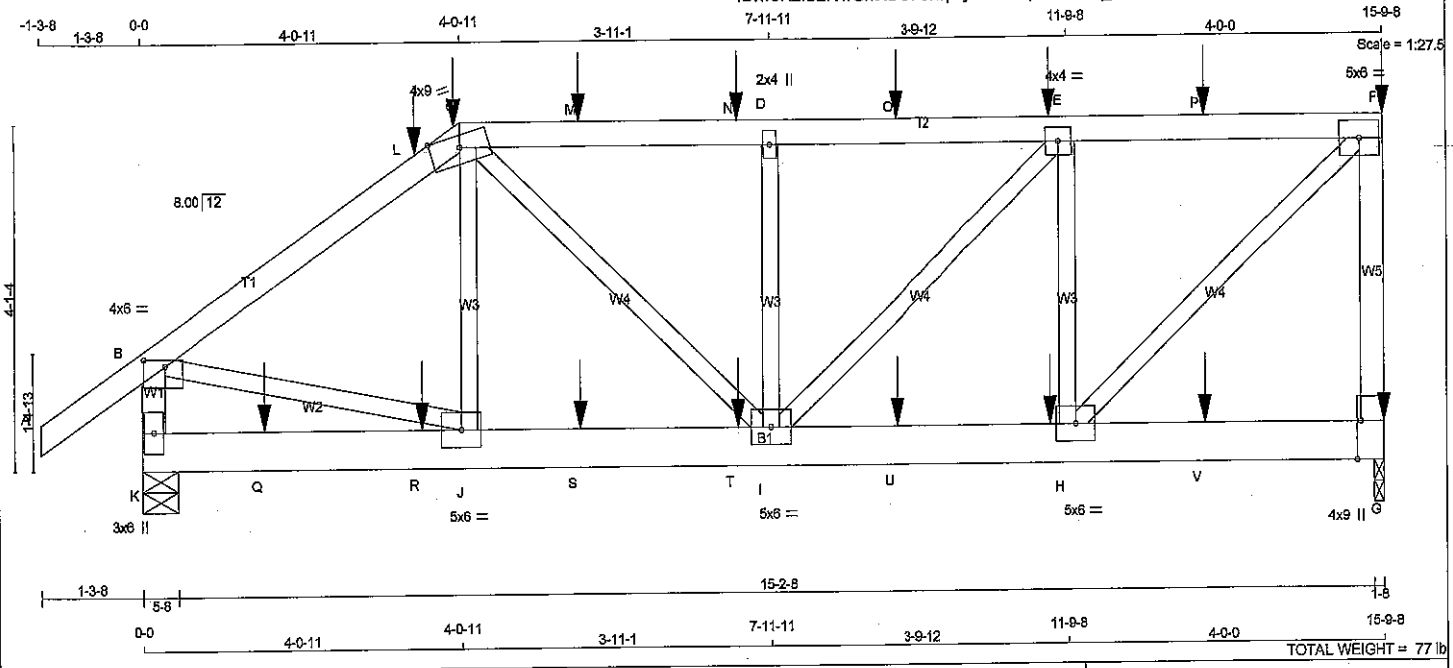


TOTAL WEIGHT = 77 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - G	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	6.0	1.00	3.25
C	TTWW-m	MT20	4.0	9.0	Edge	
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMVW-t	MT20	5.0	6.0		
G	BMV1+p	MT20	4.0	9.0	Edge	0.50
H	BMWW-t	MT20	5.0	6.0		
I	BMWWW-t	MT20	5.0	6.0		
J	BMWW-t	MT20	5.0	6.0		
K	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 244.5 lbs FACTORED DOWN AT 4-0-11, 122.8 lbs FACTORED DOWN AT 3-6-12, 101.4 lbs FACTORED DOWN AT 5-6-12, 101.4 lbs FACTORED DOWN AT 7-6-12, 101.4 lbs FACTORED DOWN AT 9-6-12, 101.4 lbs FACTORED DOWN AT 11-6-12, AND 101.4 lbs FACTORED DOWN AT 13-6-12, AND 133.9 lbs FACTORED DOWN AT 15-9-8 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 1-8-12, 69.9 lbs FACTORED DOWN AT 3-6-12, 69.9 lbs FACTORED DOWN AT 5-6-12, 69.9 lbs FACTORED DOWN AT 7-6-12, 69.9 lbs FACTORED DOWN AT 9-6-12, 69.9 lbs FACTORED DOWN AT 11-6-12, AND 69.9 lbs FACTORED DOWN AT 13-6-12, AND 68.8 lbs FACTORED DOWN AT 15-9-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1604	0	1604	0	0	1-8	1-8
K	1622	0	1622	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1321	728 / 0	305 / 0	0 / 0	0 / 0	289 / 0	0 / 0
K	1310	763 / 0	275 / 0	0 / 0	0 / 0	273 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.69 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.3	-84.3	0.12 (1)	10.00	
B- L	-1622 / 0	-84.3	-84.3	0.37 (1)	4.74	
L- C	-1622 / 0	-84.3	-84.3	0.37 (1)	4.74	
C- M	-1683 / 0	-84.3	-84.3	0.34 (1)	4.69	
M- N	-1683 / 0	-84.3	-84.3	0.34 (1)	4.69	
N- D	-1683 / 0	-84.3	-84.3	0.34 (1)	4.69	
D- O	-1683 / 0	-84.3	-84.3	0.32 (1)	4.72	
O- E	-1683 / 0	-84.3	-84.3	0.32 (1)	4.72	
E- P	-1270 / 0	-84.3	-84.3	0.29 (1)	5.32	
P- F	-1270 / 0	-84.3	-84.3	0.29 (1)	5.32	
G- F	-1487 / 0	0.0	0.0	0.37 (1)	6.68	
K- B	-1547 / 0	0.0	0.0	0.17 (1)	6.58	
K- Q	0 / 0	-28.0	-28.0	0.10 (2)	10.00	
Q- R	0 / 0	-28.0	-28.0	0.10 (2)	10.00	
R- J	0 / 0	-28.0	-28.0	0.10 (2)	10.00	
J- S	0 / 1366	-28.0	-28.0	0.23 (1)	10.00	
S- T	0 / 1366	-28.0	-28.0	0.23 (1)	10.00	
T- I	0 / 1366	-28.0	-28.0	0.23 (1)	10.00	
I- U	0 / 1270	-28.0	-28.0	0.23 (1)	10.00	
U- H	0 / 1270	-28.0	-28.0	0.23 (1)	10.00	
H- V	0 / 0	-28.0	-28.0	0.09 (2)	10.00	
V- G	0 / 0	-28.0	-28.0	0.09 (2)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- C	-105 / 224	0.06 (3)
B- J	0 / 1407	0.35 (1)
H- F	0 / 1738	0.43 (1)
C- I	0 / 437	0.11 (1)
H- E	-974 / 0	0.25 (1)
I- D	-557 / 0	0.14 (1)
I- E	0 / 578	0.14 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
C	4-0-11	-23	-25	—	FRONT	VERT	DEAD
C	4-0-11	-222	-222	—	FRONT	VERT	SNOW
E	11-6-12	-101	-101	—	BACK	VERT	TOTAL
F	15-9-8	-134	-134	—	BACK	VERT	TOTAL
G	15-9-8	-51	-89	—	BACK	VERT	TOTAL
H	11-6-12	-40	-70	—	BACK	VERT	TOTAL
L	3-6-12	-123	-123	—	BACK	VERT	TOTAL
M	5-6-12	-101	-101	—	BACK	VERT	TOTAL
N	7-6-12	-101	-101	—	BACK	VERT	TOTAL
O	9-6-12	-101	-101	—	BACK	VERT	TOTAL
P	13-6-12	-101	-101	—	BACK	VERT	TOTAL
Q	1-6-12	-40	-70	—	BACK	VERT	TOTAL
R	3-6-12	-40	-70	—	BACK	VERT	TOTAL

[M]

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.53")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.53")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.37/1.00 (B-C:1) , BC=0.23/1.00 (I-J:1) , WB=0.43/1.00 (F-H:1) , SSI=0.29/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.66 (F) (INPUT = 0.90)
JSI METAL= 0.37 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 5980-B P6/L

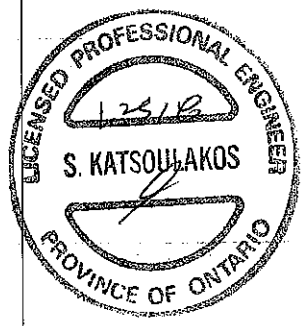
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285864	T30	1	1	TRUSS DESC.	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
S	5-6-12	-40	-70	---	BACK	VERT	TOTAL
T	7-6-12	-40	-70	---	BACK	VERT	TOTAL
U	9-6-12	-40	-70	---	BACK	VERT	TOTAL
V	13-6-12	-40	-70	---	BACK	VERT	TOTAL



DWG NO. TAM 5980-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286043	T31	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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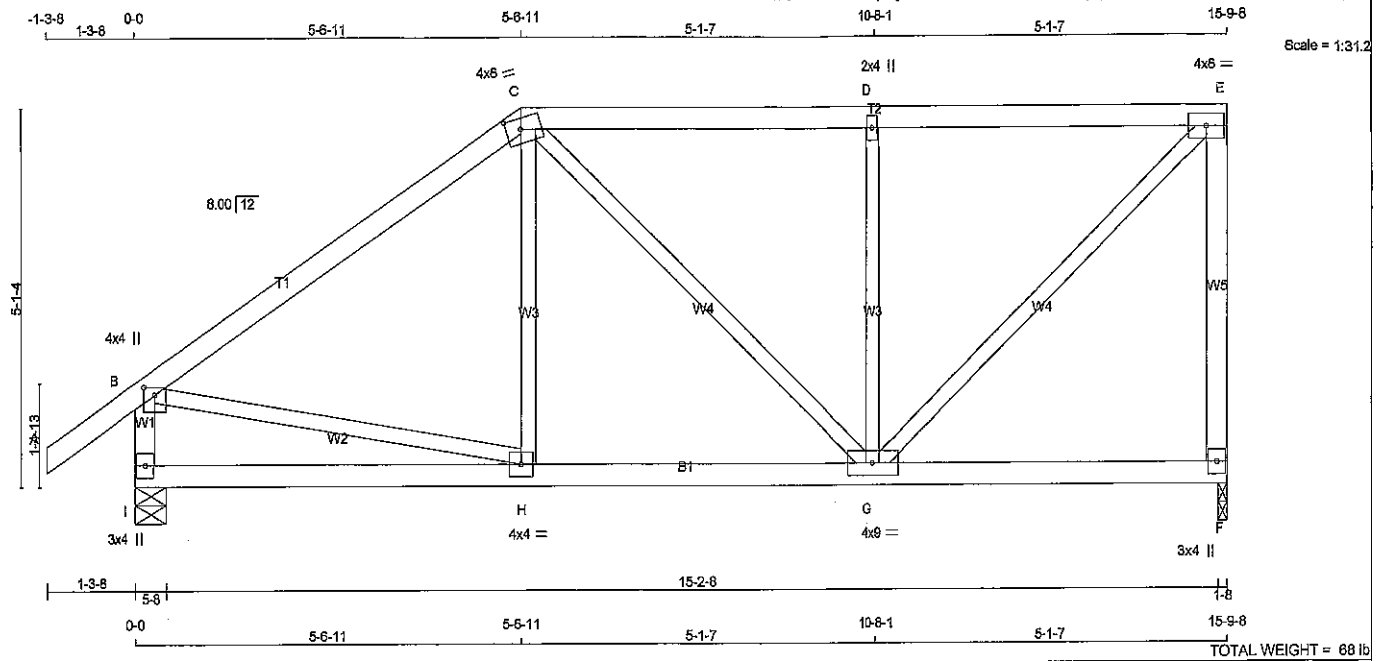

TOTAL WEIGHT = 68 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTWW-m	MT20	4.0	6.0	1.75	2.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BMWW-t	MT20	4.0	4.0		
I	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	886	0	886	0	0	1-8	1-8
I	1002	0	1002	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	728	404 / 0	166 / 0	0 / 0	0 / 0	158 / 0	0 / 0
I	806	474 / 0	166 / 0	0 / 0	0 / 0	166 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.96 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00
B- C	-821 / 0	-84.3	-84.3	0.49 (1)	5.96
C- D	-707 / 0	-84.3	-84.3	0.38 (1)	6.25
D- E	-707 / 0	-84.3	-84.3	0.38 (1)	6.25
F- E	-826 / 0	0.0	0.0	0.36 (1)	7.81
I- B	-937 / 0	0.0	0.0	0.10 (1)	7.81
I- H	0 / 0	-28.0	-28.0	0.19 (3)	10.00
H- G	0 / 883	-28.0	-28.0	0.28 (2)	10.00
G- F	0 / 0	-28.0	-28.0	0.16 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	0 / 222	0.05 (3)
C- G	-23 / 33	0.02 (3)
G- D	-532 / 0	0.20 (1)
G- E	0 / 967	0.22 (1)
B- H	0 / 895	0.16 (1)

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.53")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.53")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.49/1.00 (B-C:1) , BC=0.28/1.00 (G-H:2) , WB=0.22/1.00 (E-G:1) , SSI=0.21/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	816	354	1667	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (B) (INPUT = 0.90)
JSI METAL= 0.26 (H) (INPUT = 1.00)

DWG NO. TAM 3996 -18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286043	T32	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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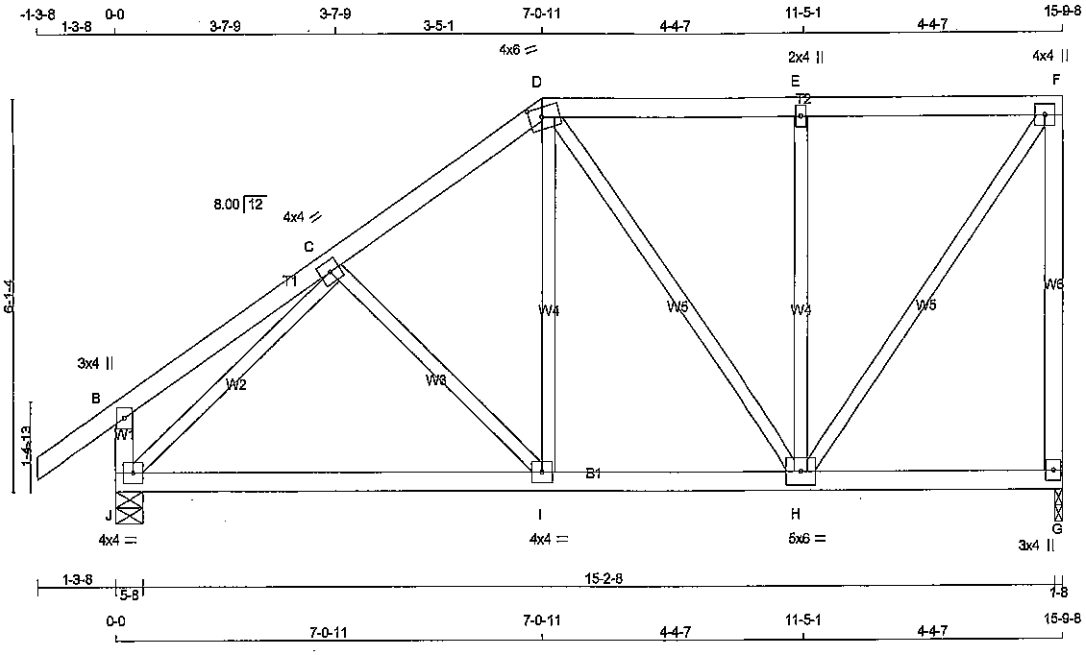


TOTAL WEIGHT = 75 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTWW-m	MT20	4.0	6.0	1.75	2.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	4.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT	REQRD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	BRG
G	886	0	886	0	0	1-8	1-8
J	1002	0	1002	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	728	404 / 0	166 / 0	0 / 0	0 / 0	158 / 0	0 / 0
J	806	474 / 0	166 / 0	0 / 0	0 / 0	168 / 0	0 / 0

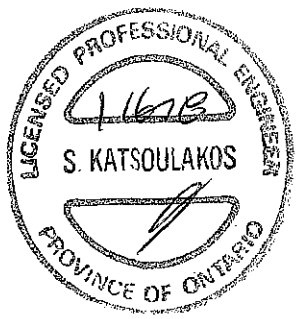
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS					WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	C- I	-127 / 52	0.05 (1)
B- C	0 / 18	-84.3	-84.3	0.16 (1)	10.00	I- D	0 / 348	0.08 (2)
C- D	-758 / 0	-84.3	-84.3	0.17 (1)	6.25	D- H	-188 / 0	0.17 (2)
D- E	-522 / 0	-84.3	-84.3	0.27 (1)	6.25	H- E	-453 / 0	0.27 (1)
E- F	-522 / 0	-84.3	-84.3	0.27 (1)	6.25	H- F	0 / 864	0.19 (1)
G- F	-831 / 0	0.0	0.0	0.60 (1)	7.81	J- C	-987 / 0	0.38 (1)
J- B	-231 / 0	0.0	0.0	0.02 (1)	7.81			
J- I	0 / 707	-28.0	-28.0	0.38 (2)	10.00			
I- H	0 / 619	-28.0	-28.0	0.38 (2)	10.00			
H- G	0 / 0	-28.0	-28.0	0.13 (2)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.53")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.53")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.60/1.00 (F-G:1) , BC=0.38/1.00 (H-I:2) , WB=0.38/1.00 (C-J:1) , SSI=0.18/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (F) (INPUT = 0.90)
JSI METAL= 0.35 (C) (INPUT = 1.00)

DWG NO. TAM 3997-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285955	T33Z6	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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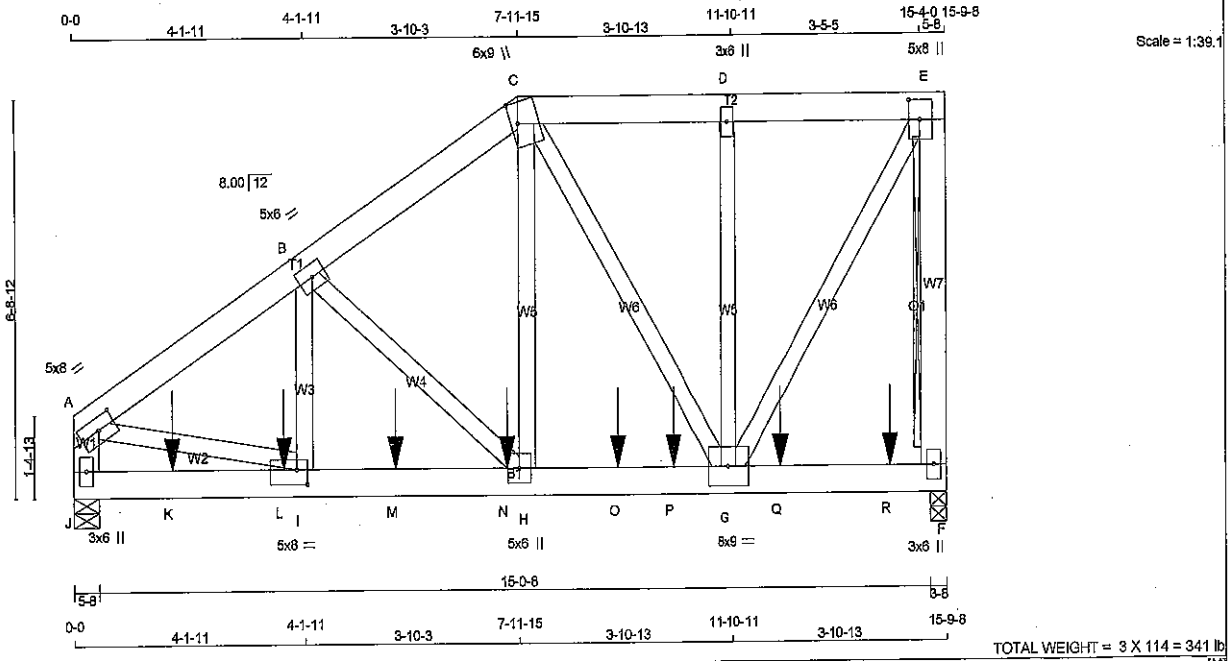


TOTAL WEIGHT = 3 X 114 = 341 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
F - E	2x6	DRY	No.2	SPF
J - A	2x6	DRY	No.2	SPF
J - F	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	TOP
C- E	2	12	TOP
E- F	2	12	TOP
J- A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
J- F	2	7	SIDE(923.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	8.0	2.50	3.75
B	TMWW-t	MT20	5.0	6.0		
C	TTWW+m	MT20	6.0	9.0	4.25	1.50
D	TMW+w	MT20	3.0	6.0		
E	TMWW+p	MT20	5.0	8.0	4.00	2.25
F	BMV1+p	MT20	3.0	6.0		
G	BMWWW-t	MT20	8.0	9.0		
H	BMWW+t	MT20	5.0	6.0		
I	BMWW-t	MT20	5.0	8.0	3.00	2.25
J	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	8352	0	8352	0	0	3-8	3-8
J	7663	0	7663	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	6781	3890 / 0	1461 / 0	0 / 0	0 / 0	1430 / 0	0 / 0
J	6195	3596 / 0	1307 / 0	0 / 0	0 / 0	1293 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.84 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x6 DRY SPF No.2 T-BRACE AT E-F

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	-8841 / 0	-84.3	-84.3	0.10 (1)		4.84
B- C	-8646 / 0	-84.3	-84.3	0.07 (1)		5.43
C- D	-4377 / 0	-84.3	-84.3	0.05 (1)		6.25
D- E	-4377 / 0	-84.3	-84.3	0.05 (1)		6.25
F- E	-7052 / 0	0.0	0.0	0.26 (1)		7.81
J- A	-6688 / 0	0.0	0.0	0.14 (1)		6.85
J- K	0 / 0	-28.0	-28.0	0.20 (1)		10.00
K- L	0 / 0	-28.0	-28.0	0.20 (1)		10.00
L- I	0 / 0	-28.0	-28.0	0.20 (1)		10.00
I- M	0 / 7342	-28.0	-28.0	0.26 (1)		10.00
M- N	0 / 7342	-28.0	-28.0	0.26 (1)		10.00
N- H	0 / 7342	-28.0	-28.0	0.26 (1)		10.00
H- O	0 / 5656	-28.0	-28.0	0.30 (1)		10.00
O- P	0 / 5656	-28.0	-28.0	0.30 (1)		10.00
P- G	0 / 5656	-28.0	-28.0	0.30 (1)		10.00
G- Q	0 / 328	-28.0	-28.0	0.21 (1)		10.00
Q- R	0 / 328	-28.0	-28.0	0.21 (1)		10.00
R- F	0 / 328	-28.0	-28.0	0.21 (1)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI
I- B	0 / 2559	0.14 (1)
B- H	-2457 / 0	0.27 (1)
H- C	0 / 5425	0.29 (1)
C- G	-2455 / 0	0.55 (1)
G- D	-269 / 22	0.04 (1)
G- E	0 / 7981	0.43 (1)
A- I	0 / 7512	0.40 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
K	1-9-4	-1826	-1826	---	BACK	VERT	TOTAL
L	3-9-4	-1826	-1826	---	BACK	VERT	TOTAL
M	5-9-4	-1826	-1826	---	BACK	VERT	TOTAL
N	7-9-4	-1826	-1826	---	BACK	VERT	TOTAL
O	9-9-4	-1826	-1826	---	BACK	VERT	TOTAL
P	10-9-4	-1712	-1712	---	BACK	VERT	TOTAL
Q	12-9-4	-1712	-1712	---	BACK	VERT	TOTAL
R	14-9-4	-1712	-1712	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.53")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.53")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.28/1.00 (E-F:1) , BC=0.30/1.00 (G-H:1) , WB=0.55/1.00 (C-G:1) , SSI=0.56/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

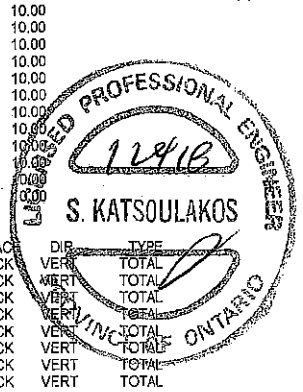
NAIL VALUES

PLATE	GRIP(ORY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (E) (INPUT = 0.90)
JSI METAL= 0.56 (A) (INPUT = 1.00)



P6 1/2

DWG NO. TAM 5114 -S5
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285955	T33Z6	1	3	TRUSS DESC.	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1826.2 lbs FACTORED DOWN AT 1-9-4, 1826.2 lbs FACTORED DOWN AT 3-9-4, 1826.2 lbs FACTORED DOWN AT 5-9-4, 1826.2 lbs FACTORED DOWN AT 7-9-4, 1826.2 lbs FACTORED DOWN AT 9-9-4, 1711.9 lbs FACTORED DOWN AT 10-9-4, AND 1711.9 lbs FACTORED DOWN AT 12-9-4, AND 1711.9 lbs FACTORED DOWN AT 14-9-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 5114 -18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
286039	T37	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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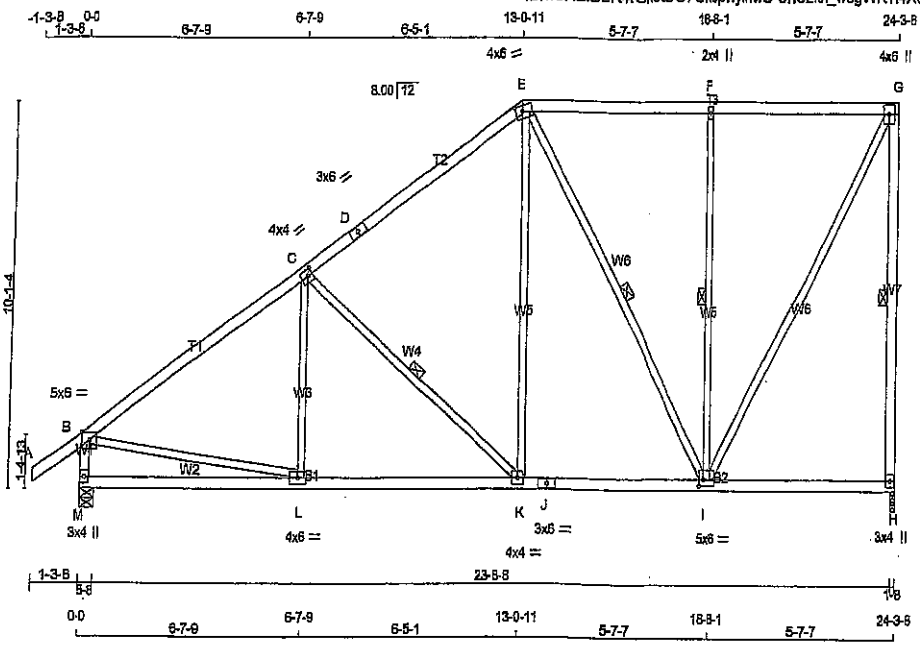


Scale = 1:59.4

TOTAL WEIGHT = 2 X 122 = 243 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	4.0	6.0	1.75	2.50
F	TMW+w	MT20	2.0	4.0		
G	TMWW+p	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWWW-t	MT20	5.0	6.0	2.00	1.50
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	1363	0	1363	0	0	1-8	1-8
M	1479	0	1479	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1119	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
M	1196	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.69 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

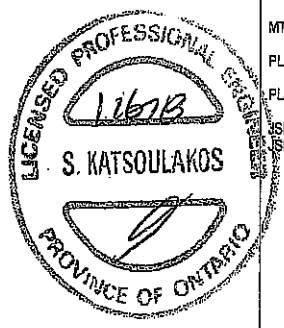
1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-H, C-K, E-I, F-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00
B-C	-1522 / 0	-84.3	-84.3	0.55 (1)	4.69
C-D	-1051 / 0	-84.3	-84.3	0.49 (1)	5.48
D-E	-1051 / 0	-84.3	-84.3	0.49 (1)	5.46
E-F	-644 / 0	-84.3	-84.3	0.34 (1)	6.25
F-G	-644 / 0	-84.3	-84.3	0.34 (1)	6.25
H-G	-1299 / 0	0.0	0.0	0.85 (1)	5.66
M-B	-1404 / 0	0.0	0.0	0.14 (1)	6.67
M-L	0 / 0	-28.0	-28.0	0.31 (3)	10.00
L-K	0 / 1296	-28.0	-28.0	0.46 (2)	10.00
K-J	0 / 848	-28.0	-28.0	0.31 (2)	10.00
J-I	0 / 848	-28.0	-28.0	0.31 (2)	10.00
I-H	0 / 0	-28.0	-28.0	0.21 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L-C	-1 / 267	0.06 (3)
C-K	-594 / 0	0.30 (1)
K-E	0 / 545	0.12 (2)
E-I	-413 / 0	0.40 (1)
I-F	-586 / 0	0.41 (1)
I-G	0 / 1269	0.29 (1)
B-L	0 / 1314	0.30 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.55/1.00 (G-H:1) , BC=0.48/1.00 (K-L:2) , WB=0.41/1.00 (F-I:1) , SSI=0.23/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.60

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SSI GRIP= 0.83 (L) (INPUT = 0.90)
SSI METAL= 0.32 (L) (INPUT = 1.00)

DWG NO. TAM3974-13
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285956	T40	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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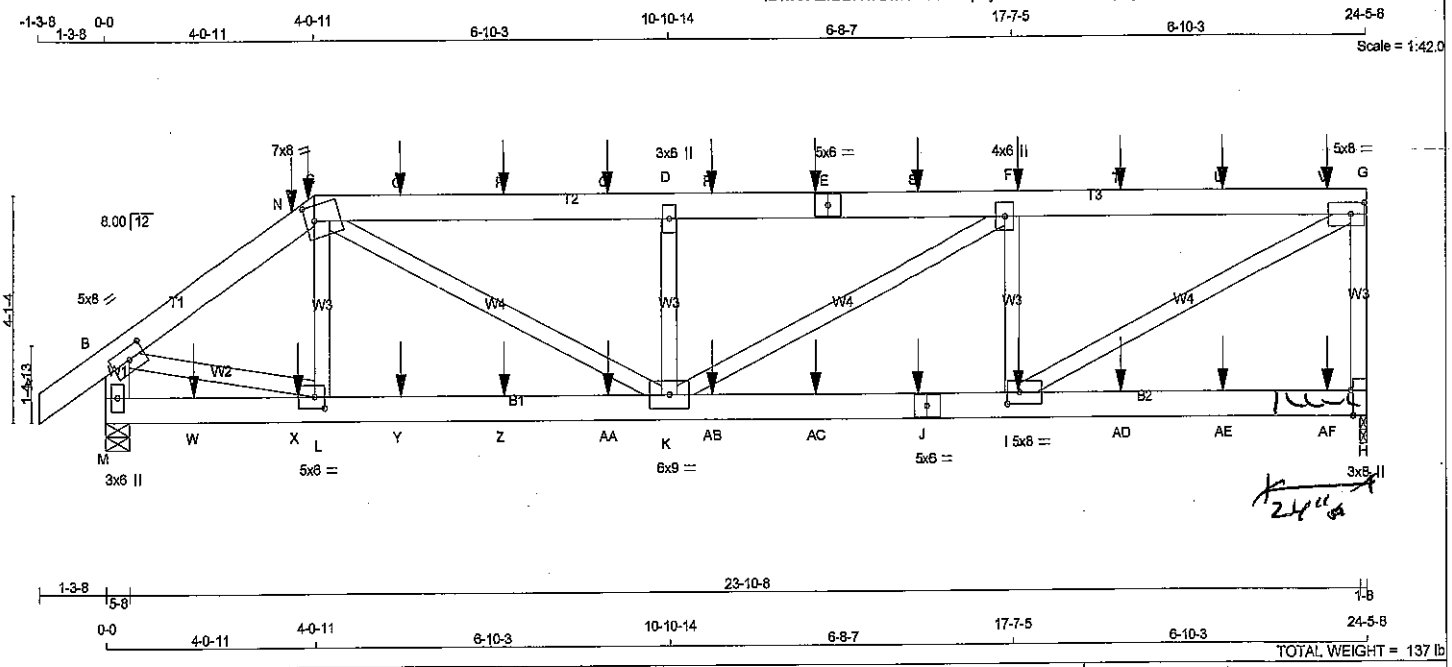

TOTAL WEIGHT = 137 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - G	2x6	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.50	3.75
C	TTWW-m	MT20	7.0	8.0	3.25	2.00
D	TMW+w	MT20	3.0	6.0		
E	TS-t	MT20	5.0	6.0		
F	TMWW+t	MT20	4.0	6.0		
G	TMVW-t	MT20	5.0	8.0	2.50	3.00
H	BMV1+p	MT20	3.0	8.0	Edge	0.50
I	BMWW-t	MT20	5.0	8.0	2.50	3.00
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	6.0	9.0		
L	BMWW-t	MT20	5.0	8.0	2.50	2.25
M	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 244.5 lbs FACTORED DOWN AT 4-0-11, 129.8 lbs FACTORED DOWN AT 3-8-12, 101.4 lbs FACTORED DOWN AT 5-8-12, 101.4 lbs FACTORED DOWN AT 7-8-12, 101.4 lbs FACTORED DOWN AT 9-8-12, 101.4 lbs FACTORED DOWN AT 11-8-12, 101.4 lbs FACTORED DOWN AT 13-8-12, 101.4 lbs FACTORED DOWN AT 15-8-12, 101.4 lbs FACTORED DOWN AT 17-8-12, 101.4 lbs FACTORED DOWN AT 19-8-12, AND 101.4 lbs FACTORED DOWN AT 21-8-12, AND 112.8 lbs FACTORED DOWN AT 23-8-12 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 1-8-12, 69.9 lbs FACTORED DOWN AT 3-8-12, 69.9 lbs FACTORED DOWN AT 5-8-12, 69.9 lbs FACTORED DOWN AT 7-8-12, 69.9 lbs FACTORED DOWN AT 9-8-12, 69.9 lbs FACTORED DOWN AT 11-8-12, 69.9 lbs FACTORED DOWN AT 13-8-12, 69.9 lbs FACTORED DOWN AT 15-8-12, 69.9 lbs FACTORED DOWN AT 17-8-12, 69.9 lbs FACTORED DOWN AT 19-8-12, AND 69.9 lbs FACTORED DOWN AT 21-8-12, AND 76.6 lbs FACTORED DOWN AT 23-8-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GRDSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	2309	0	2309	0	0	1-8	4+48
M	2439	0	2439	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEA0	SOIL
H	1907	1040 / 0	447 / 0	0 / 0	0 / 0	420 / 0	0 / 0
M	1982	1134 / 0	429 / 0	0 / 0	0 / 0	42D / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.72 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF) CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 33	-84.3	-84.3	0.07 (1)	10.00
B- N	-2810 / 0	-84.3	-84.3	0.19 (1)	4.80
N- C	-2810 / 0	-84.3	-84.3	0.19 (1)	4.80
C- O	-4018 / 0	-84.3	-84.3	0.56 (1)	3.72
O- P	-4018 / 0	-84.3	-84.3	0.56 (1)	3.72
P- Q	-4018 / 0	-84.3	-84.3	0.56 (1)	3.72
Q- D	-4018 / 0	-84.3	-84.3	0.56 (1)	3.72
D- R	-4018 / 0	-84.3	-84.3	0.55 (1)	3.72
R- E	-4018 / 0	-84.3	-84.3	0.55 (1)	3.72
E- S	-4018 / 0	-84.3	-84.3	0.55 (1)	3.72
S- F	-4018 / 0	-84.3	-84.3	0.55 (1)	3.72
F- T	-3301 / 0	-84.3	-84.3	0.53 (1)	4.08
T- U	-3301 / 0	-84.3	-84.3	0.53 (1)	4.08
U- V	-3301 / 0	-84.3	-84.3	0.53 (1)	4.08
V- G	-3301 / 0	-84.3	-84.3	0.53 (1)	4.08
H- G	-2160 / 0	0.0	0.0	0.50 (1)	5.72
M- B	-2386 / 0	0.0	0.0	0.17 (1)	6.62
M- W	0 / 0	-28.0	-28.0	0.19 (2)	10.00
W- X	0 / 0	-28.0	-28.0	0.19 (2)	10.00
X- L	0 / 0	-28.0	-28.0	0.19 (2)	10.00
L- Y	0 / 2349	-28.0	-28.0	0.42 (1)	10.00
Y- Z	0 / 2349	-28.0	-28.0	0.42 (1)	10.00
Z-AA	0 / 2349	-28.0	-28.0	0.42 (1)	10.00
AA- K	0 / 2349	-28.0	-28.0	0.42 (1)	10.00
K-AB	0 / 3301	-28.0	-28.0	0.57 (2)	10.00
AB-AC	0 / 3301	-28.0	-28.0	0.57 (2)	10.00
AC- J	0 / 3301	-28.0	-28.0	0.57 (2)	10.00
J- I	0 / 3301	-28.0	-28.0	0.57 (2)	10.00
I-AD	0 / 0	-28.0	-28.0	0.26 (3)	10.00
AD-AE	0 / 0	-28.0	-28.0	0.26 (3)	10.00
AE-AF	0 / 0	-28.0	-28.0	0.26 (3)	10.00
AF- H	0 / 0	-28.0	-28.0	0.28 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- C	-197 / 265	0.05 (3)
C- K	0 / 1688	0.33 (1)
K- D	-972 / 0	0.17 (1)
K- F	0 / 818	0.14 (1)
I- F	-1395 / 0	0.24 (1)
I- G	0 / 3739	0.66 (1)
B- L	0 / 2410	0.43 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	4-0-11	-23	-25	---	FRONT	VERT	DEAD
C	4-0-11	-222	-222	---	FRONT	VERT	SNOW
E	13-8-12	-101	-101	---	BACK	VERT	TOTAL

[M]

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.8	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.82")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.82")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.56/1.00 (C-D:1) , BC=0.57/1.00 (I-K:2) , WB=0.66/1.00 (G-I:1) , SSI=0.35/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1557	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.62 (G) (INPUT = 1.00)

PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM5982-15
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285956	T40	1	1	TRUSS DESC.	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
F	17-8-12	-101	-101	—	BACK	VERT	TOTAL
I	17-8-12	-40	-70	—	BACK	VERT	TOTAL
J	15-8-12	-40	-70	—	BACK	VERT	TOTAL
N	3-8-12	-130	-130	—	BACK	VERT	TOTAL
O	5-8-12	-101	-101	—	BACK	VERT	TOTAL
P	7-8-12	-101	-101	—	BACK	VERT	TOTAL
Q	9-8-12	-101	-101	—	BACK	VERT	TOTAL
R	11-8-12	-101	-101	—	BACK	VERT	TOTAL
S	15-8-12	-101	-101	—	BACK	VERT	TOTAL
T	19-8-12	-101	-101	—	BACK	VERT	TOTAL
U	21-8-12	-101	-101	—	BACK	VERT	TOTAL
V	23-8-12	-113	-113	—	BACK	VERT	TOTAL
W	1-8-12	-40	-70	—	BACK	VERT	TOTAL
X	3-8-12	-40	-70	—	BACK	VERT	TOTAL
Y	5-8-12	-40	-70	—	BACK	VERT	TOTAL
Z	7-8-12	-40	-70	—	BACK	VERT	TOTAL
AA	9-8-12	-40	-70	—	BACK	VERT	TOTAL
AB	11-8-12	-40	-70	—	BACK	VERT	TOTAL
AC	13-8-12	-40	-70	—	BACK	VERT	TOTAL
AD	19-8-12	-40	-70	—	BACK	VERT	TOTAL
AE	21-8-12	-40	-70	—	BACK	VERT	TOTAL
AF	23-8-12	-44	-77	—	BACK	VERT	TOTAL

SCAB JOINT #H WITH 2 X 6 #2
SPF 24" LONG 1 SIDE(S) USING 2
ROW(S) 3/4" COMMON WIRE NAILS
AT 2" O/C STAGGERED (18 NAILS/SCAB)
REQUIRED FOR 1 1/2" BRG PROVISION.

LICENSED PROFESSIONAL ENGINEER
1/25/18
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 5982.18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285815	T41	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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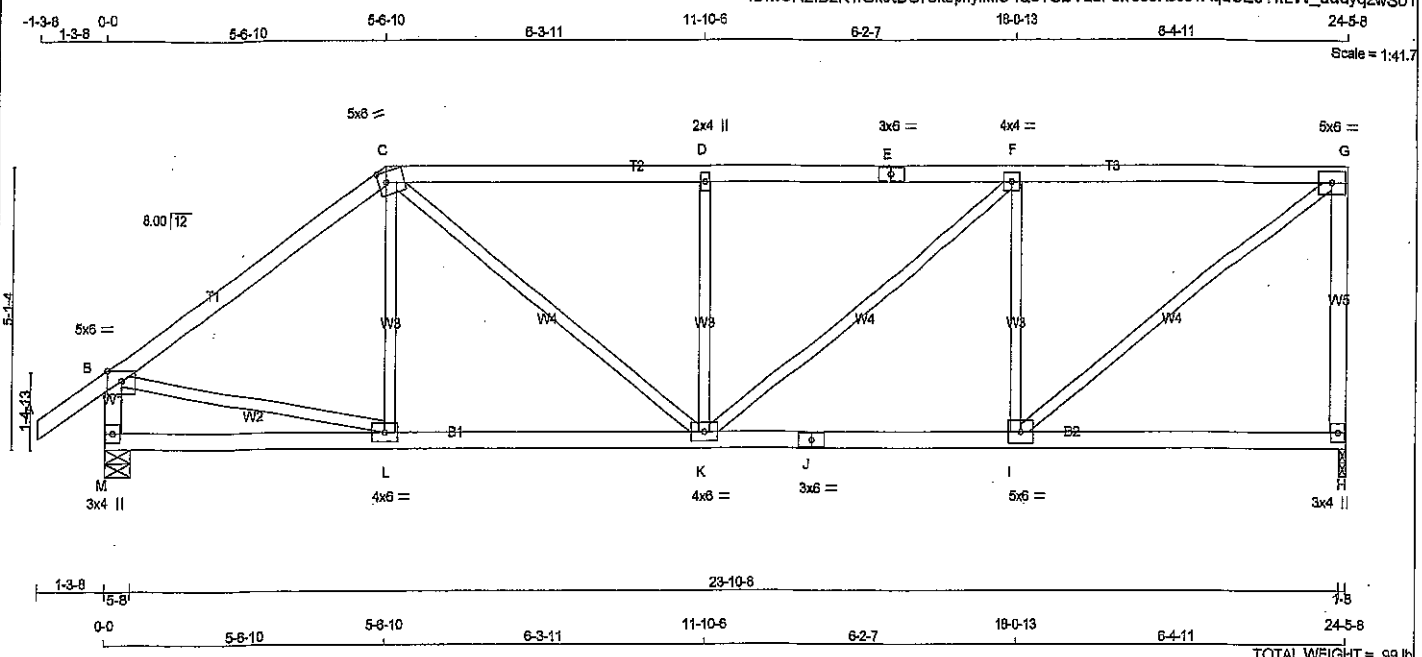

TOTAL WEIGHT = 99 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TTWW-m	MT20	5.0	6.0	2.25	1.75
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	1373	0	1373	0	0	1-8	1-8
M	1468	0	1468	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1127	626 / 0	257 / 0	0 / 0	0 / 0	245 / 0	0 / 0
M	1206	696 / 0	257 / 0	0 / 0	0 / 0	253 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.30 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	L- C	-50 / 204	0.05 (3)
B- C	-1507 / 0	-84.3	-84.3	0.55 (1)	4.68	B- L	0 / 1275	0.29 (1)
C- D	-1822 / 0	-84.3	-84.3	0.58 (1)	4.33	I- G	0 / 1799	0.40 (1)
D- E	-1822 / 0	-84.3	-84.3	0.58 (1)	4.30	C- K	0 / 717	0.16 (1)
E- F	-1822 / 0	-84.3	-84.3	0.58 (1)	4.30	I- F	-882 / 0	0.34 (1)
F- G	-1437 / 0	-84.3	-84.3	0.54 (1)	4.77	K- D	-571 / 0	0.22 (1)
H- G	-1300 / 0	0.0	0.0	0.57 (1)	7.08	K- F	0 / 487	0.11 (1)
M- B	-1427 / 0	0.0	0.0	0.15 (1)	6.84			
M- L	0 / 0	-28.0	-28.0	0.24 (3)	10.00			
L- K	0 / 1251	-28.0	-28.0	0.39 (2)	10.00			
K- J	0 / 1437	-28.0	-28.0	0.44 (2)	10.00			
J- I	0 / 1437	-28.0	-28.0	0.44 (2)	10.00			
I- H	0 / 0	-28.0	-28.0	0.28 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.82")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.82")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.58/1.00 (D-F:1) , BC=0.44/1.00 (I-K:2) , WB=0.40/1.00 (G-I:1) , SSI=0.25/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL= 0.44 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
1/13/18
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM3363.18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285956	T41A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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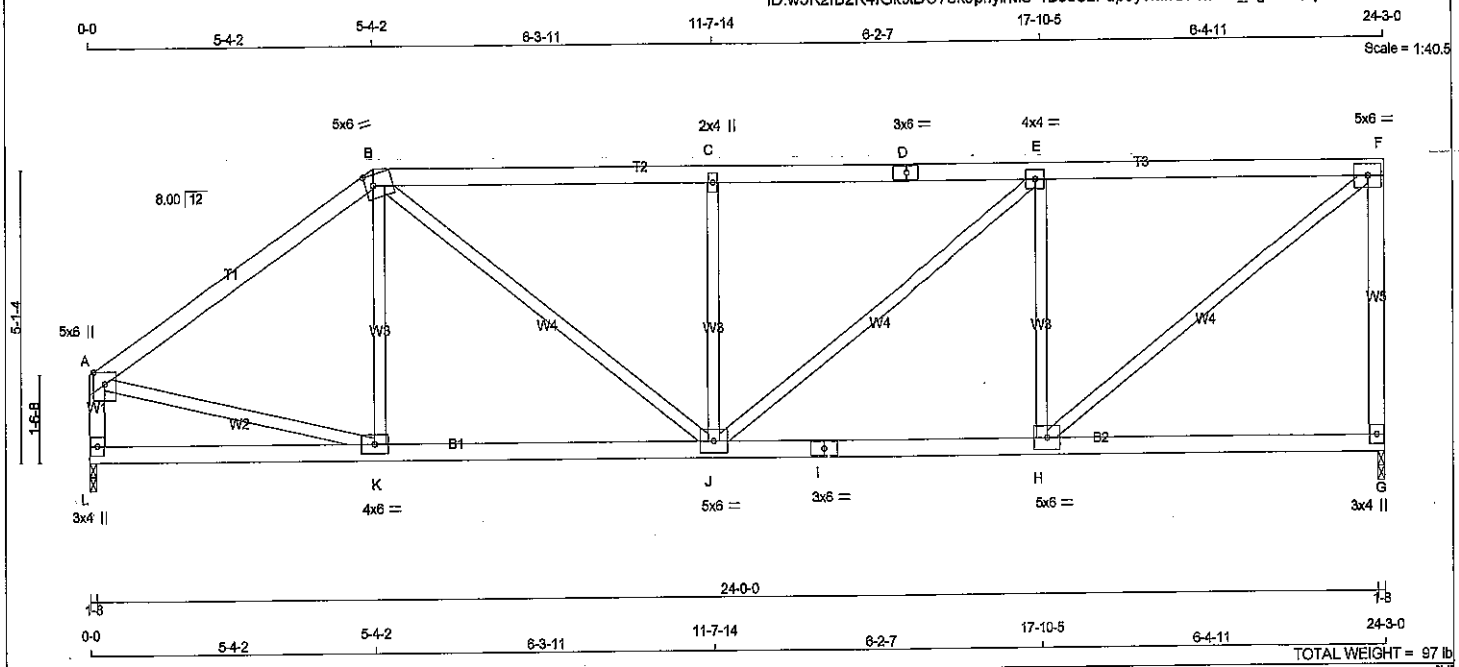

TOTAL WEIGHT = 97 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - A	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	Edge	
B	TTWW-m	MT20	5.0	6.0	2.25	1.75
C	TMW+w	MT20	2.0	4.0		
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMVW-t	MT20	5.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	5.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	5.0	6.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1361	0	1361	0	0	1-8	1-8
L	1361	0	1361	0	0	1-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1118	620 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
L	1118	620 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.33 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-1451 / 0	-84.3	-84.3	0.50 (1)	4.82	K- B	-85 / 161	0.04 (3)
B- C	-1791 / 0	-84.3	-84.3	0.56 (1)	4.36	A- K	0 / 1236	0.28 (1)
C- D	-1792 / 0	-84.3	-84.3	0.57 (1)	4.33	H- F	0 / 1780	0.40 (1)
D- E	-1792 / 0	-84.3	-84.3	0.57 (1)	4.33	B- J	0 / 738	0.17 (1)
E- F	-1422 / 0	-84.3	-84.3	0.54 (1)	4.79	H- E	-870 / 0	0.33 (1)
G- F	-1289 / 0	0.0	0.0	0.57 (1)	7.10	J- C	-571 / 0	0.22 (1)
L- A	-1302 / 0	0.0	0.0	0.14 (1)	7.08	J- E	0 / 468	0.11 (1)
L- K	0 / 0	-28.0	-28.0	0.23 (3)	10.00			
K- J	0 / 1204	-28.0	-28.0	0.38 (2)	10.00			
J- I	0 / 1422	-28.0	-28.0	0.44 (2)	10.00			
I- H	0 / 1422	-28.0	-28.0	0.44 (2)	10.00			
H- G	0 / 0	-28.0	-28.0	0.28 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010 .

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.57/1.00 (C-E:1) , BC=0.44/1.00 (H-J:2) , WB=0.40/1.00 (F-H:1) , SSI=0.25/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1687	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (K) (INPUT = 0.90)
JSI METAL= 0.44 (I) (INPUT = 1.00)

DWG NO. TAM 5483-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285815	T42A	1	1	TRUSS DESC.	

Tamarack Roof Trusses, Burlington

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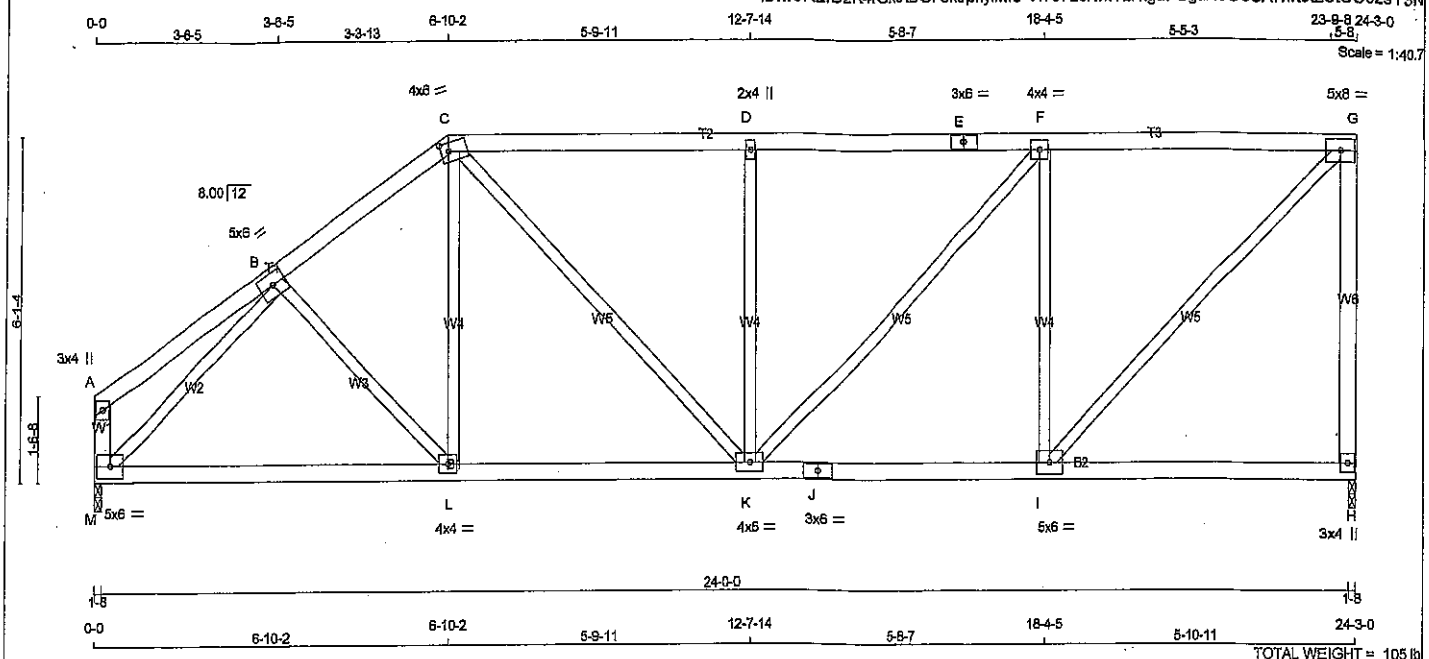

TOTAL WEIGHT = 105 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - A	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	5.0	6.0		
C	TTWW-m	MT20	4.0	6.0	1.75	1.75
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1361	0	1361	0	0	1-8	1-8
M	1361	0	1361	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1118	620 / 0	255 / 0	0 / 0	0 / 0	242 / 0	0 / 0
M	1118	620 / 0	255 / 0	0 / 0	0 / 0	242 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.85 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI
A- B	0 / 17	-84.3	-84.3	0.14 (1)	10.00	B- L	0 / 134	0.03 (3)	
B- C	-1430 / 0	-84.3	-84.3	0.17 (1)	5.26	L- C	0 / 279	0.06 (2)	
C- D	-1471 / 0	-84.3	-84.3	0.44 (1)	4.86	M- B	-1633 / 0	0.82 (1)	
D- E	-1471 / 0	-84.3	-84.3	0.45 (1)	4.85	I- G	0 / 1558	0.35 (1)	
E- F	-1471 / 0	-84.3	-84.3	0.45 (1)	4.85	C- K	0 / 415	0.09 (1)	
F- G	-1109 / 0	-84.3	-84.3	0.43 (1)	5.43	I- F	-906 / 0	0.53 (1)	
H- G	-1295 / 0	0.0	0.0	0.93 (1)	7.10	K- D	-525 / 0	0.31 (1)	
M- A	-114 / 0	0.0	0.0	0.01 (1)	7.61	K- F	0 / 517	0.12 (1)	
M- L	0 / 1141	-28.0	-28.0	0.43 (2)	10.00				
L- K	0 / 1177	-28.0	-28.0	0.45 (2)	10.00				
K- J	0 / 1109	-28.0	-28.0	0.37 (2)	10.00				
J- I	0 / 1109	-28.0	-28.0	0.37 (2)	10.00				
I- H	0 / 0	-28.0	-28.0	0.24 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.93/1.00 (G-H:1) , BC=0.45/1.00 (K-L:2) , WB=0.62/1.00 (B-M:1) , SSI=0.23/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (C) (INPUT = 0.90)
JSI METAL= 0.40 (B) (INPUT = 1.00)

DWG NO. TAM 5073 -18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285815	T43A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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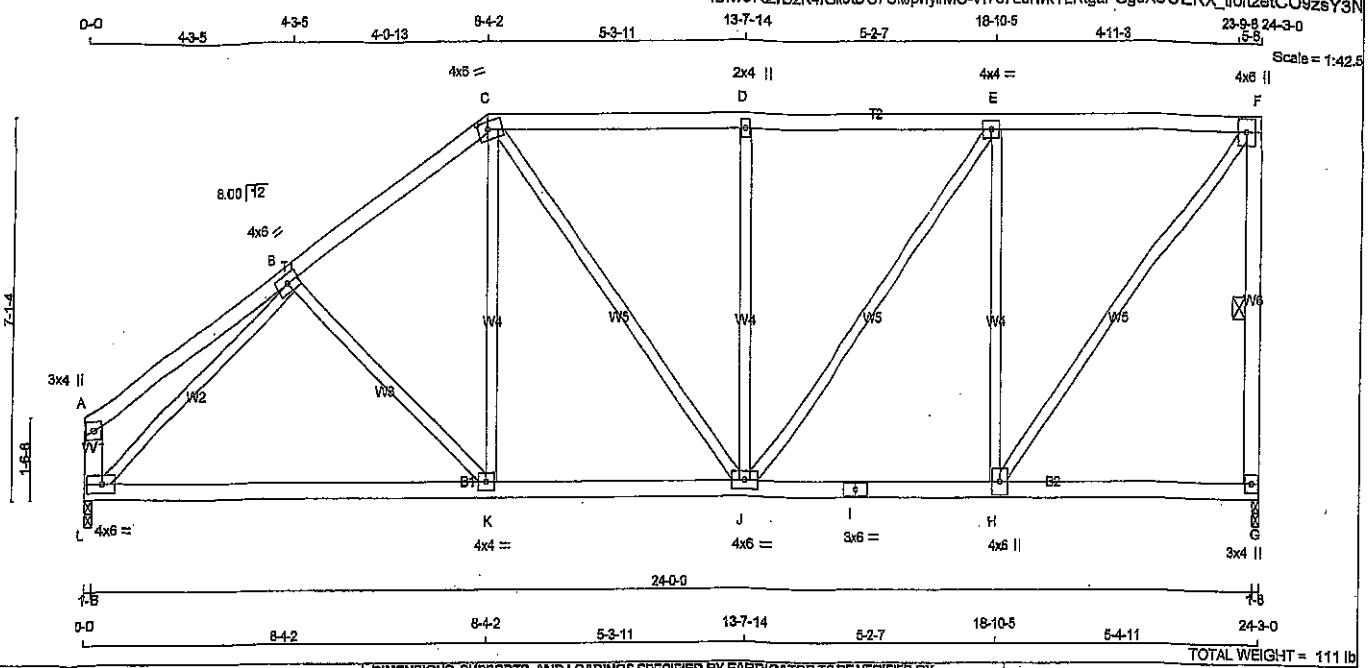

TOTAL WEIGHT = 111 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - A	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (tablels in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	6.0		
C	TTWW-m	MT20	4.0	6.0	1.75	2.50
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMWW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWW+-t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWV+	MT20	4.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMW1--t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1361	0	1361	0	0	1-8	1-8
L	1361	0	1361	0	0	1-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1118	620 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
L	1118	620 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 22	-84.3	-84.3	0.23 (1)	10.00
B- C	-1369 / 0	-84.3	-84.3	0.27 (1)	5.25
C- D	-1227 / 0	-84.3	-84.3	0.35 (1)	5.36
D- E	-1227 / 0	-84.3	-84.3	0.35 (1)	5.33
E- F	-864 / 0	-84.3	-84.3	0.34 (1)	6.05
G- F	-1302 / 0	0.0	0.0	0.30 (1)	5.65
L- A	-136 / 0	0.0	0.0	0.01 (1)	7.81
L- K	0 / 1176	-28.0	-28.0	0.56 (2)	10.00
K- J	0 / 1124	-28.0	-28.0	0.57 (2)	10.00
J- I	0 / 884	-28.0	-28.0	0.32 (2)	10.00
I- H	0 / 884	-28.0	-28.0	0.32 (2)	10.00
H- G	0 / 0	-28.0	-28.0	0.21 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- K	-77 / 99	0.05 (1)
K- C	0 / 387	0.09 (2)
L- B	-1629 / 0	0.90 (1)
H- F	0 / 1425	0.32 (1)
C- J	0 / 168	0.04 (1)
H- E	-940 / 0	0.82 (1)
J- D	-479 / 0	0.42 (1)
J- E	0 / 555	0.13 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.21")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 853 (0.34")

CSI: TC=0.36/1.00 (D-E:1) , BC=0.57/1.00 (J-K:2) , WB=0.90/1.00 (B-L:1) , SSI=0.21/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.40 (B) (INPUT = 1.00)

DWG NO. TAM5074 -B
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285815	T44A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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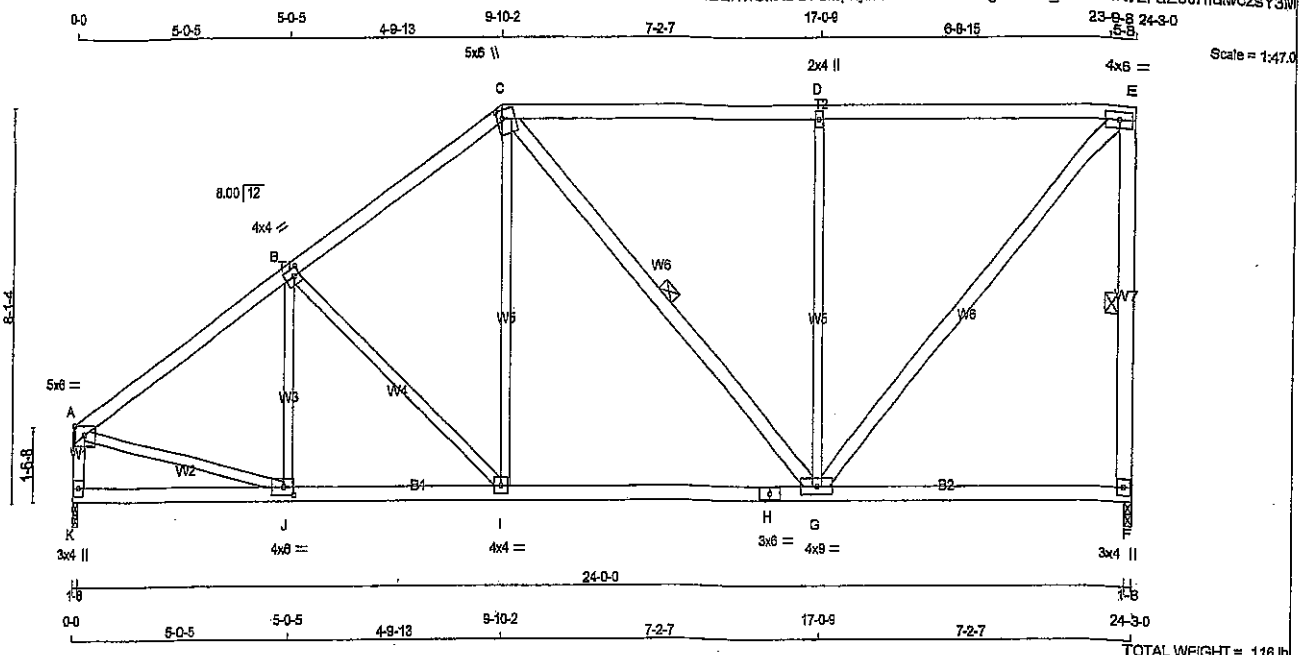

TOTAL WEIGHT = 116 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	6.0	Edge	
B	TMWW-t	MT20	4.0	4.0	2.00	1.50
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMWW-t	MT20	4.0	6.0	2.00	2.75
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	1361	0	1361	0	0	1-8	1-8
K	1361	0	1361	0	0	1-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1116	620 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
K	1116	620 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.74 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-F, C-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)		MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO			FR-TO		
A-B	-1477 / 0	-84.3	-84.3	0.40 (1)	4.93	J-B	-159 / 110	0.06 (1)
B-C	-1264 / 0	-84.3	-84.3	0.38 (1)	5.24	B-I	-307 / 0	0.26 (1)
C-D	-973 / 0	-84.3	-84.3	0.83 (1)	4.74	I-C	0 / 419	0.09 (2)
D-E	-973 / 0	-84.3	-84.3	0.83 (1)	4.74	C-G	-87 / 0	0.05 (1)
F-E	-1281 / 0	0.0	0.0	0.38 (1)	5.70	G-D	-754 / 0	0.95 (1)
K-A	-1301 / 0	0.0	0.0	0.14 (1)	7.08	G-E	0 / 1431	0.23 (1)
						A-J	0 / 1288	0.29 (1)
K-J	0 / 0	-26.0	-26.0	0.15 (3)	10.00			
J-I	0 / 1252	-26.0	-26.0	0.33 (2)	10.00			
I-H	0 / 1032	-26.0	-26.0	0.50 (2)	10.00			
H-G	0 / 1032	-26.0	-26.0	0.50 (2)	10.00			
G-F	0 / 0	-26.0	-26.0	0.38 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.83/1.00 (D-E:1) , BC=0.50/1.00 (G-I:2) , WB=0.95/1.00 (D-G:1) , SSI=0.30/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (E) (INPUT = 0.90)
JSI METAL= 0.41 (H) (INPUT = 1.00)

DWG NO. TAM 5075 -18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285956	T45A	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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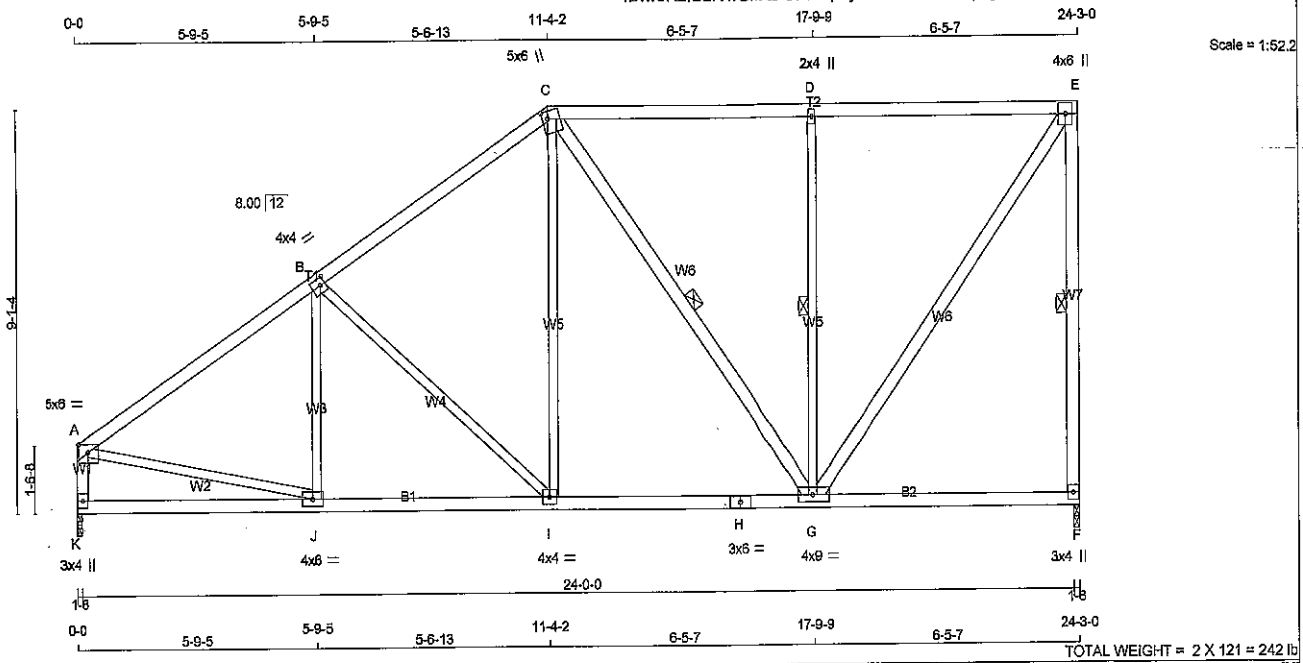

TOTAL WEIGHT = 2 X 121 = 242 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	6.0	Edge	
B	TMWW-t	MT20	4.0	4.0	2.00	1.50
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW+p	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQR0 BRG IN-SX
F	1361	0	1361	0	0	1-8	1-8
K	1361	0	1361	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1118	620 / 0	255 / 0	0 / 0	0 / 0	242 / 0	0 / 0
K	1118	620 / 0	255 / 0	0 / 0	0 / 0	242 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.70 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, C-G, D-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1466 / 0	-84.3	-84.3	0.56 (1)	4.70
B- C	-1161 / 0	-84.3	-84.3	0.52 (1)	5.20
C- D	-796 / 0	-84.3	-84.3	0.63 (1)	5.66
D- E	-796 / 0	-84.3	-84.3	0.63 (1)	5.66
F- E	-1288 / 0	0.0	0.0	0.50 (1)	5.69
K- A	-1295 / 0	0.0	0.0	0.14 (1)	7.10
K- J	0 / 0	-28.0	-28.0	0.22 (3)	10.00
J- I	0 / 1262	-28.0	-28.0	0.37 (2)	10.00
I- H	0 / 943	-26.0	-28.0	0.41 (2)	10.00
H- G	0 / 943	-26.0	-28.0	0.41 (2)	10.00
G- F	0 / 0	-26.0	-28.0	0.30 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- B	-89 / 183	0.04 (3)
B- I	-435 / 0	0.51 (1)
I- C	0 / 475	0.11 (2)
C- G	-249 / 0	0.16 (1)
G- D	-675 / 0	0.36 (1)
G- E	0 / 1343	0.22 (1)
A- J	0 / 1290	0.29 (1)

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.63/1.00 (D-E:1) , BC=0.41/1.00 (G-I:2) , WB=0.51/1.00 (B-I:1) , SSI=0.26/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (G) (INPUT = 0.90)
JSI METAL= 0.32 (J) (INPUT = 1.00)

DWG NO. TAM 5984-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285956	T46A	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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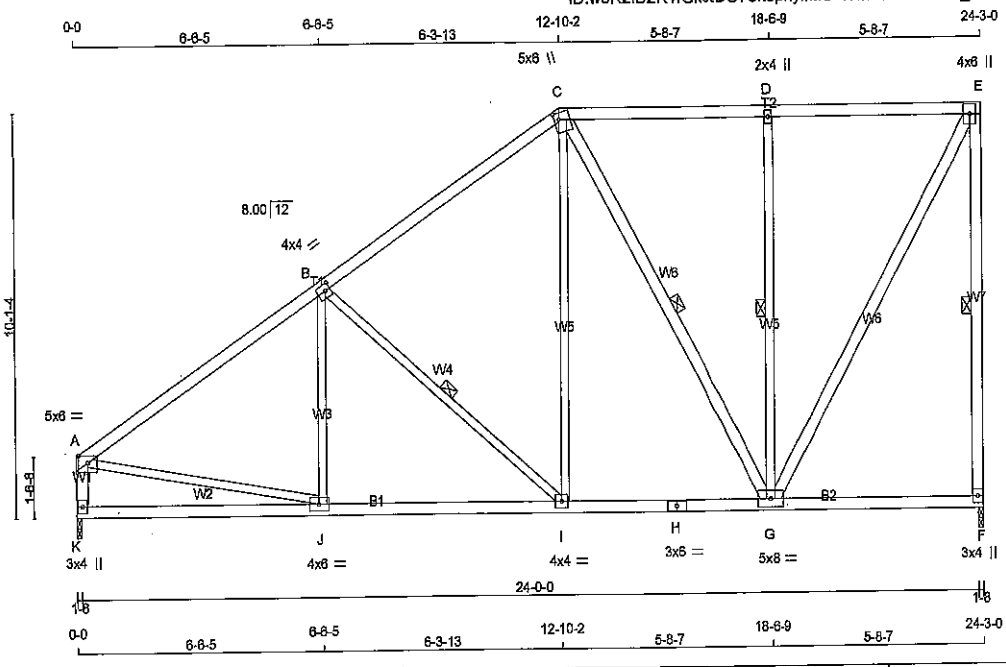

TOTAL WEIGHT = 3 X 127 = 381 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	6.0	Edge	
B	TMWW-t	MT20	4.0	4.0	2.00	1.50
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW+p	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	8.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1361	0	1361	0	0	1-8	1-8
K	1361	0	1361	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1118	620 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
K	1118	620 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, B-I, C-G, D-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	-1481 / 0	-84.3	-84.3	0.52 (1)	4.77	
B- C	-1050 / 0	-84.3	-84.3	0.48 (1)	5.50	
C- D	-651 / 0	-84.3	-84.3	0.35 (1)	6.25	
D- E	-651 / 0	-84.3	-84.3	0.35 (1)	6.25	
F- E	-1296 / 0	0.0	0.0	0.65 (1)	5.67	
K- A	-1288 / 0	0.0	0.0	0.13 (1)	7.11	
K- J	0 / 0	-28.0	-28.0	0.30 (3)	10.00	
J- I	0 / 1262	-28.0	-28.0	0.44 (2)	10.00	
I- H	0 / 847	-28.0	-28.0	0.32 (2)	10.00	
H- G	0 / 847	-28.0	-28.0	0.32 (2)	10.00	
G- F	0 / 0	-28.0	-28.0	0.22 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI
J- B	-29 / 248	0.06 (3)
B- I	-557 / 0	0.27 (1)
I- C	0 / 531	0.12 (2)
C- G	-393 / 0	0.28 (1)
G- D	-595 / 0	0.42 (1)
G- E	0 / 1288	0.21 (1)
A- J	0 / 1284	0.29 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.65/1.00 (E-F:1) , BC=0.44/1.00 (I-J:2) , WB=0.42/1.00 (D-G:1) , SSI=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (J) (INPUT = 0.90)
JSI METAL= 0.32 (J) (INPUT = 1.00)

DWG NO. TAM 5985-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285956	T47	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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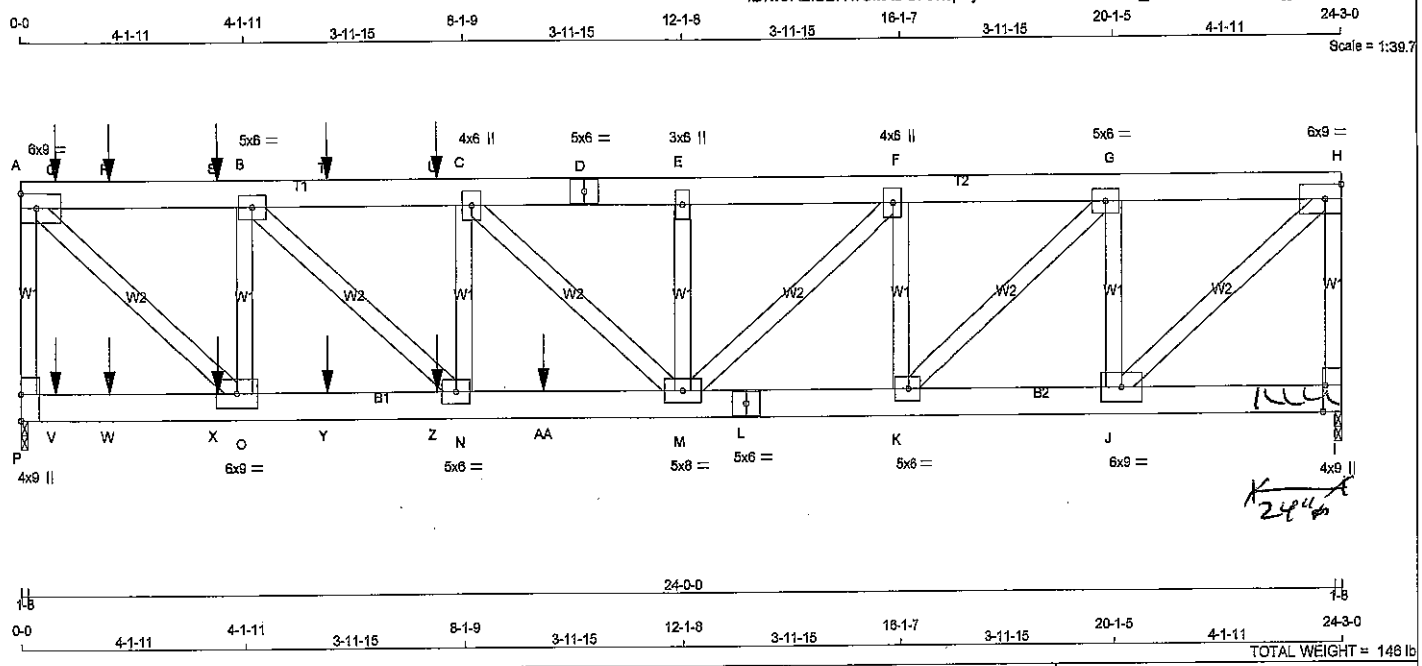


LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
P - A	2x4	DRY	No.2	SPF
A - D	2x6	DRY	2100F 1.8E	SPF
D - H	2x6	DRY	2100F 1.8E	SPF
I - H	2x4	DRY	No.2	SPF
P - L	2x6	DRY	2100F 1.8E	SPF
L - I	2x6	DRY	2100F 1.8E	SPF
ALL WEBS	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-t	MT20	6.0	9.0		Edge
B	TMWW-t	MT20	5.0	6.0		
C	TMWW+t	MT20	4.0	6.0		
D	TS-t	MT20	5.0	6.0		
E	TMW+w	MT20	3.0	6.0		
F	TMWW+t	MT20	4.0	6.0		
G	TMWW-t	MT20	5.0	6.0		
H	TMWW-t	MT20	6.0	9.0		Edge
I	BMV1+p	MT20	4.0	9.0	Edge	0.50
J	BMWW-t	MT20	6.0	9.0		
K	BMWW-t	MT20	5.0	6.0		
L	BS-t	MT20	5.0	6.0		
M	BMWWW-t	MT20	5.0	8.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMWW-t	MT20	6.0	9.0		
P	BMV1+p	MT20	4.0	9.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 116.3 lbs FACTORED DOWN AT 7-12, 101.4 lbs FACTORED DOWN AT 1-7-12, 101.4 lbs FACTORED DOWN AT 3-7-12, AND 101.4 lbs FACTORED DOWN AT 5-7-12, AND 101.4 lbs FACTORED DOWN AT 7-7-12 ON TOP CHORD, AND 78.6 lbs FACTORED DOWN AT 7-12, 69.9 lbs FACTORED DOWN AT 1-7-12, 69.9 lbs FACTORED DOWN AT 3-7-12, 69.9 lbs FACTORED DOWN AT 5-7-12, AND 69.9 lbs FACTORED DOWN AT 7-7-12, AND 963.1 lbs FACTORED DOWN AT 9-7-0 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

SCAB JOINT # P ea WITH 2 X 6 #2 SPF 24" LONG 1 SIDE(S) USING 2 ROW(S) 3¼" COMMON WIRE NAILS AT 2" o/c STAGGERED (20 NAILS/SCAB) REQUIRED FOR 1½" BRG PROVISION.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	2558	0	2558	0	0	1-8	1-8
I	1854	0	1854	0	0	1-8	2-6

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	2123	1141 / 0	509 / 0	0 / 0	0 / 0	473 / 0	0 / 0
I	1532	835 / 0	360 / 0	0 / 0	0 / 0	338 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.15 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
P- A	-2425 / 0	0.0	0.0	0.56 (1)	5.43	
A- Q	-2417 / 0	-84.3	-84.3	0.12 (1)	6.16	
Q- R	-2417 / 0	-84.3	-84.3	0.12 (1)	6.16	
R- S	-2417 / 0	-84.3	-84.3	0.12 (1)	6.16	
S- B	-2417 / 0	-84.3	-84.3	0.12 (1)	6.16	
B- T	-3903 / 0	-84.3	-84.3	0.12 (1)	5.15	
T- U	-3903 / 0	-84.3	-84.3	0.12 (1)	5.15	
U- C	-3903 / 0	-84.3	-84.3	0.12 (1)	5.15	
C- D	-3969 / 0	-84.3	-84.3	0.08 (1)	5.15	
D- E	-3969 / 0	-84.3	-84.3	0.08 (1)	5.15	
E- F	-3969 / 0	-84.3	-84.3	0.08 (1)	5.16	
F- G	-3118 / 0	-84.3	-84.3	0.08 (1)	5.67	
G- H	-1876 / 0	-84.3	-84.3	0.06 (1)	6.25	
I- H	-1802 / 0	0.0	0.0	0.41 (1)	6.18	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
A- O	0 / 3221	0.57 (1)
O- B	-2010 / 0	0.34 (1)
B- N	0 / 2013	0.36 (1)
N- C	-487 / 0	0.06 (1)
C- M	0 / 88	0.02 (1)
M- E	-329 / 0	0.06 (1)
M- F	0 / 1152	0.20 (1)
K- F	-1108 / 0	0.19 (1)
K- G	0 / 1681	0.30 (1)
J- G	-1513 / 0	0.26 (1)
J- H	0 / 2500	0.44 (1)

P- V	0 / 0	-28.0	-28.0	0.07 (2)	10.00
V- W	0 / 0	-28.0	-28.0	0.07 (2)	10.00
W- X	0 / 0	-28.0	-28.0	0.07 (2)	10.00
X- O	0 / 0	-28.0	-28.0	0.07 (2)	10.00
O- Y	0 / 2417	-28.0	-28.0	0.24 (1)	10.00
Y- Z	0 / 2417	-28.0	-28.0	0.24 (1)	10.00
Z- N	0 / 2417	-28.0	-28.0	0.24 (1)	10.00
N-AA	0 / 3903	-28.0	-28.0	0.48 (1)	10.00
AA- M	0 / 3903	-28.0	-28.0	0.48 (1)	10.00
M- L	0 / 3118	-28.0	-28.0	0.24 (1)	10.00
L- K	0 / 3118	-28.0	-28.0	0.24 (1)	10.00
K- J	0 / 1876	-28.0	-28.0	0.13 (1)	10.00
J- I	0 / 0	-28.0	-28.0	0.03 (2)	10.00

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
Q	7-12	-116	-116	---	FRONT	VERT	TOTAL
R	1-7-12	-101	-101	---	FRONT	VERT	TOTAL
S	3-7-12	-101	-101	---	FRONT	VERT	TOTAL
T	5-7-12	-101	-101	---	FRONT	VERT	TOTAL
U	7-7-12	-101	-101	---	FRONT	VERT	TOTAL
V	7-12	-45	-79	---	FRONT	VERT	TOTAL
W	1-7-12	-40	-70	---	FRONT	VERT	TOTAL
X	3-7-12	-40	-70	---	FRONT	VERT	TOTAL

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.56/1.00 (A-P:1) , BC=0.48/1.00 (M-N:1) , WB=0.57/1.00 (A-O:1) , SSI=0.46/1.00 (M-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (K) (INPUT = 0.90)
JSI METAL= 0.77 (L) (INPUT = 1.00)

PROFESSIONAL ENGINEER PROVINCE OF ONTARIO S. KATSOULAKOS 1/25/18

DWG NO. TAM 5996 -18
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

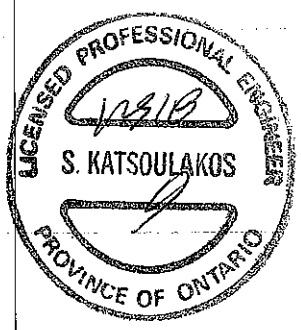
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285956	T47	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:w3K2IB2K4fGk9tDO7SkJphylfMC-YNfEhJGXaP4m_xnJbQsPsBhr1FP_4ExiHjtgPizru0U

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
Y	5-7-12	-40	-70	—	FRONT	VERT	TOTAL
Z	7-7-12	-40	-70	—	FRONT	VERT	TOTAL
AA	9-7-0	-963	-963	—	FRONT	VERT	TOTAL



DWG NO. TAM 5986 .18 PGM

STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285952	T49X	6	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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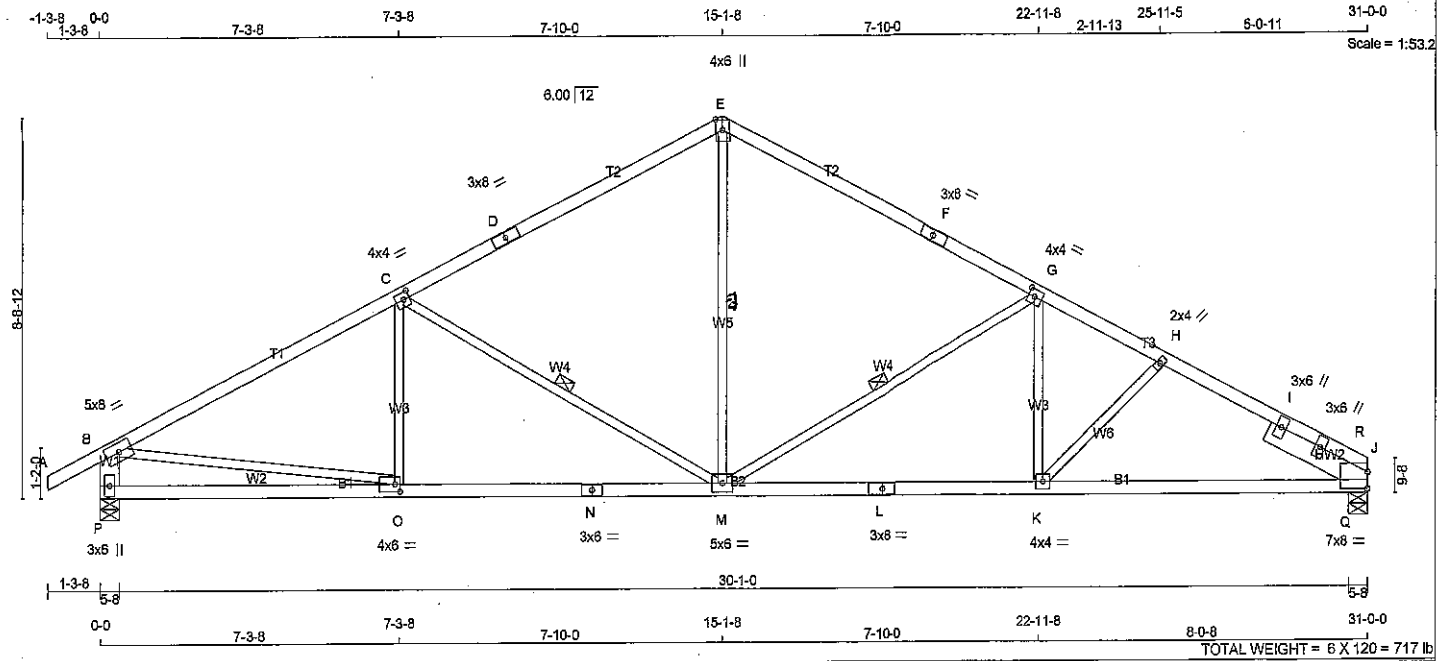


TOTAL WEIGHT = 6 X 120 = 717 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - J	2x4	DRY	No.2	SPF
P - B	2x6	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
REINFORCING MEMBERS				
HW2	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	8.0		
E	TTW+p	MT20	4.0	6.0	Edge	
F	TS-t	MT20	3.0	8.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
H	TMW+w	MT20	2.0	4.0		
J	TMBMR1-I	MT20	7.0	8.0	4.50	
J	RT+t	MT20	3.0	6.0		
J	RT+t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	8.0		
M	BMWWW-t	MT20	5.0	6.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	6.0	2.00	1.50
P	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	1854	0	1854	0	0	5-8	5-8
J	1740	0	1740	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1508	863 / 0	326 / 0	0 / 0	0 / 0	318 / 0	0 / 0
J	1429	793 / 0	326 / 0	0 / 0	0 / 0	310 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.41 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-M, G-M.

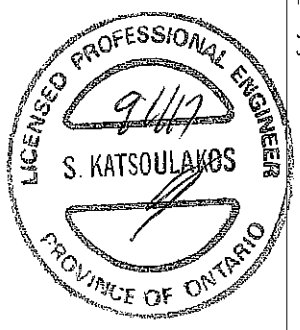
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 26	-84.3	-84.3	0.11 (1)	10.00
B- C	-2552 / 0	-84.3	-84.3	0.83 (1)	3.41
C- D	-1842 / 0	-84.3	-84.3	0.79 (1)	3.93
D- E	-1842 / 0	-84.3	-84.3	0.79 (1)	3.93
E- F	-1838 / 0	-84.3	-84.3	0.75 (1)	4.03
F- G	-1838 / 0	-84.3	-84.3	0.75 (1)	4.03
G- H	-2627 / 0	-84.3	-84.3	0.60 (1)	3.55
H- I	-2019 / 0	-84.3	-84.3	0.35 (1)	4.43
I- R	-2019 / 0	-84.3	-84.3	0.35 (1)	4.43
R- J	-2363 / 0	-84.3	-84.3	0.13 (1)	4.38
P- B	-1771 / 0	0.0	0.0	0.11 (1)	7.48
P- O	0 / 0	-28.0	-28.0	0.37 (3)	10.00
O- N	0 / 2314	-28.0	-28.0	0.64 (2)	10.00
N- M	0 / 2314	-28.0	-28.0	0.64 (2)	10.00
M- L	0 / 2435	-28.0	-28.0	0.63 (2)	10.00
L- K	0 / 2435	-28.0	-28.0	0.63 (2)	10.00
K- Q	0 / 2370	-28.0	-28.0	0.58 (2)	10.00
Q- J	0 / 1281	-28.0	-28.0	0.48 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- C	-43 / 276	0.06 (3)
C- M	-803 / 0	0.46 (1)
M- E	0 / 1096	0.25 (1)
M- G	-943 / 0	0.54 (1)
K- G	0 / 331	0.07 (3)
B- O	0 / 2330	0.52 (1)
K- H	0 / 92	0.02 (1)
Q- R	0 / 1434	0.00 (1)
Q- I	-1506 / 0	0.12 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.30")

CSI: TC=0.83 (B-C:1) , BC=0.64 (M-O:2) , WB=0.54 (G-M:1) , SSI=0.35 (J-Q:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (O) (INPUT = 0.90)
JSI METAL= 0.80 (L) (INPUT = 1.00)

DWG NO. TAM45593-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285952	T93	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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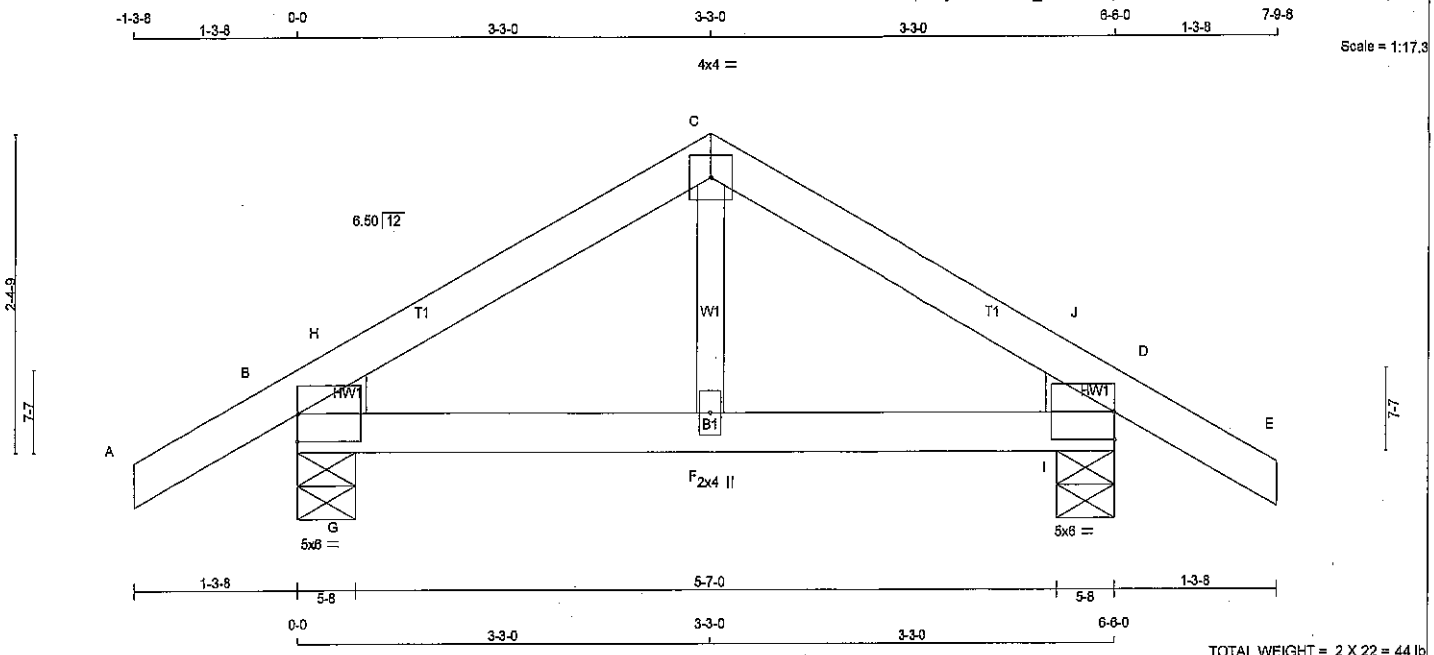

TOTAL WEIGHT = 2 X 22 = 44 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMBH1-I	MT20	5.0	6.0	
C	TTW-p	MT20	4.0	4.0	
D	TMBH1-I	MT20	5.0	6.0	
F	BMW+w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG	HEEL
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	WEDGE
B	480	0	480	0	0	5-8	5-8	2x4 L
D	480	0	480	0	0	5-8	5-8	2x4 R

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	377	236 / 0	68 / 0	0 / 0	0 / 0	73 / 0	0 / 0
D	377	236 / 0	68 / 0	0 / 0	0 / 0	73 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 17	-84.3	-84.3	0.11 (1)	10.00
B- H	-412 / 0	-84.3	-84.3	0.05 (1)	6.25
H- C	-355 / 0	-84.3	-84.3	0.08 (1)	6.25
C- J	-355 / 0	-84.3	-84.3	0.08 (1)	6.25
J- D	-412 / 0	-84.3	-84.3	0.05 (1)	6.25
D- E	0 / 17	-84.3	-84.3	0.11 (1)	10.00
B- G	0 / 305	-28.0	-28.0	0.08 (1)	10.00
G- F	0 / 305	-28.0	-28.0	0.10 (1)	10.00
F- I	0 / 305	-28.0	-28.0	0.10 (1)	10.00
I- D	0 / 305	-28.0	-28.0	0.08 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	0 / 165	0.04 (2)
G- H	-34 / 113	0.00 (1)
I- J	-34 / 113	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.11 (D-E:1) , BC=0.10 (F-G:1) , WB=0.04 (C-F:2) , SSI=0.09 (C-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (D) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM45594-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285952	T93Z	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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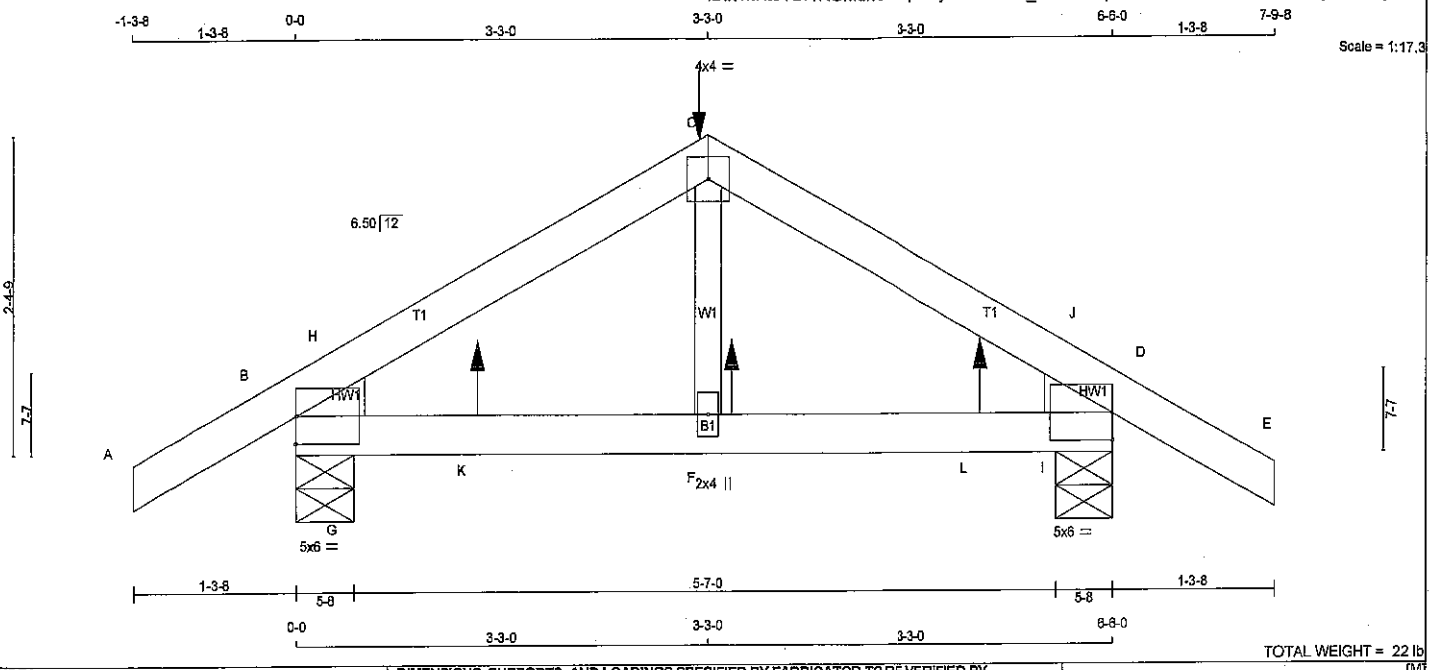

TOTAL WEIGHT = 22 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMBH1-I	MT20	5.0	6.0	
C	TTW-p	MT20	4.0	4.0	
D	TMBH1-I	MT20	5.0	6.0	
F	BMW+w	MT20	2.0	4.0	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1.7 lbs FACTORED DOWN AND 105.9 lbs FACTORED UP AT 3-3-0 ON TOP CHORD, AND 16.7 lbs FACTORED UP AT 1-5-4, AND 16.7 lbs FACTORED UP AT 3-5-4, AND 16.7 lbs FACTORED UP AT 5-5-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
B	464	0	464	0	0	5-8	5-8	2x4 L
D	462	0	462	0	0	5-8	5-8	2x4 R

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	367	225 / 0	69 / 0	0 / 0	0 / 0	73 / 0	0 / 0
D	366	224 / 0	69 / 0	0 / 0	0 / 0	73 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 17	-84.3	-84.3	0.12 (1)	10.00	F- C	0 / 162	0.04 (7)
B- H	-383 / 0	-84.3	-84.3	0.07 (1)	6.25	G- H	-95 / 113	0.00 (1)
H- C	-336 / 0	-84.3	-84.3	0.10 (5)	6.25	I- J	-94 / 113	0.00 (1)
C- J	-336 / 0	-84.3	-84.3	0.10 (5)	6.25			
J- D	-383 / 0	-84.3	-84.3	0.07 (1)	6.25			
D- E	0 / 17	-84.3	-84.3	0.12 (1)	10.00			
B- G	0 / 289	-28.0	-28.0	0.10 (5)	10.00			
G- K	0 / 289	-28.0	-28.0	0.11 (1)	10.00			
K- F	0 / 289	-28.0	-28.0	0.11 (1)	10.00			
F- L	0 / 289	-28.0	-28.0	0.11 (1)	10.00			
L- I	0 / 289	-28.0	-28.0	0.11 (1)	10.00			
I- D	0 / 289	-28.0	-28.0	0.09 (5)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	3-3-0	-1	-2	106	BACK	VERT	TOTAL
F	3-5-4	11	---	17	BACK	VERT	TOTAL
K	1-5-4	11	---	17	BACK	VERT	TOTAL
L	5-5-4	11	---	17	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % DF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.12 (D-E:1) , BC=0.11 (F-G:1) , WB=0.04 (C-F:7) , SSI=0.10 (C-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (D) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
9162
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM45595-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285953	T112	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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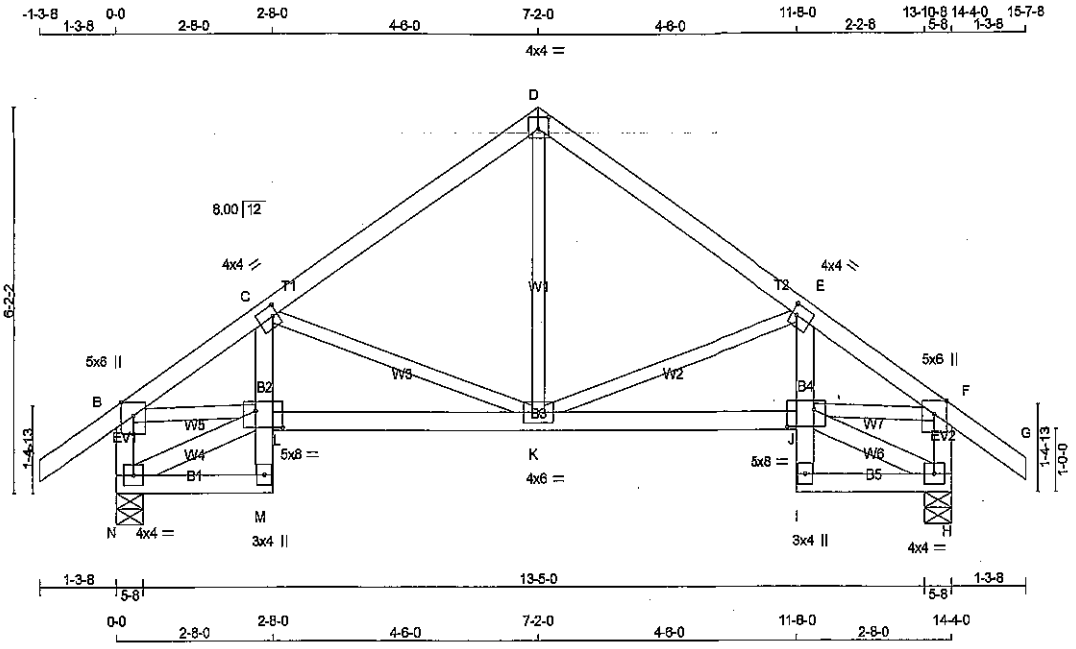

TOTAL WEIGHT = 2 X 68 = 136 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
M - C	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
I - E	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMVW-t	MT20	4.0	4.0	2.00	1.00
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMVW-t	MT20	4.0	4.0	2.00	1.00
F	TMVW+p	MT20	5.0	6.0	Edge	
H	BMVW1-t	MT20	4.0	4.0		
I	BMV+p	MT20	3.0	4.0		
J	BVMWW-I	MT20	5.0	8.0	3.25	5.50
K	BMWWW-t	MT20	4.0	8.0		
L	BVMWW-I	MT20	5.0	8.0	3.25	5.50
M	BMV+p	MT20	3.0	4.0		
N	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	920	0	920	0	0	5-8	5-8
H	920	0	920	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	739	437 / 0	150 / 0	0 / 0	0 / 0	152 / 0	0 / 0
H	739	437 / 0	150 / 0	0 / 0	0 / 0	152 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.60 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 32	-84.3	-84.3	1	0.11 (1)	10.00	K-D	0 / 482	0.11 (1)
B-C	-1234 / 0	-84.3	-84.3	1	0.17 (1)	5.60	K-E	-500 / 0	0.20 (1)
C-D	-748 / 0	-84.3	-84.3	1	0.22 (1)	6.25	C-K	-500 / 0	0.20 (1)
D-E	-748 / 0	-84.3	-84.3	1	0.22 (1)	6.25	N-L	-40 / 0	0.00 (1)
E-F	-1234 / 0	-84.3	-84.3	1	0.17 (1)	5.60	B-L	0 / 1053	0.24 (1)
F-G	0 / 32	-84.3	-84.3	1	0.11 (1)	10.00	J-H	-40 / 0	0.00 (1)
N-B	-869 / 0	0.0	0.0	1	0.09 (1)	7.81	J-F	0 / 1053	0.24 (1)
H-F	-869 / 0	0.0	0.0	1	0.09 (1)	7.81			
N-M	0 / 37	-28.0	-28.0	2	0.06 (2)	10.00			
M-L	0 / 62	0.0	0.0	1	0.08 (1)	10.00			
L-C	0 / 147	0.0	0.0	1	0.07 (1)	10.00			
L-K	0 / 1076	-28.0	-28.0	2	0.30 (2)	10.00			
K-J	0 / 1076	-28.0	-28.0	2	0.30 (2)	10.00			
I-J	0 / 62	0.0	0.0	1	0.08 (1)	10.00			
J-E	0 / 147	0.0	0.0	1	0.07 (1)	10.00			
I-H	0 / 37	-28.0	-28.0	2	0.06 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.46")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.46")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.22/1.00 (D-E:1) , BC=0.30/1.00 (J-K:2) , WB=0.24/1.00 (F-J:1) , SSI=0.15/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (L) (INPUT = 0.90)
JSI METAL= 0.24 (F) (INPUT = 1.00)

DWG NO. TAM-21223-T12
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285952	T122S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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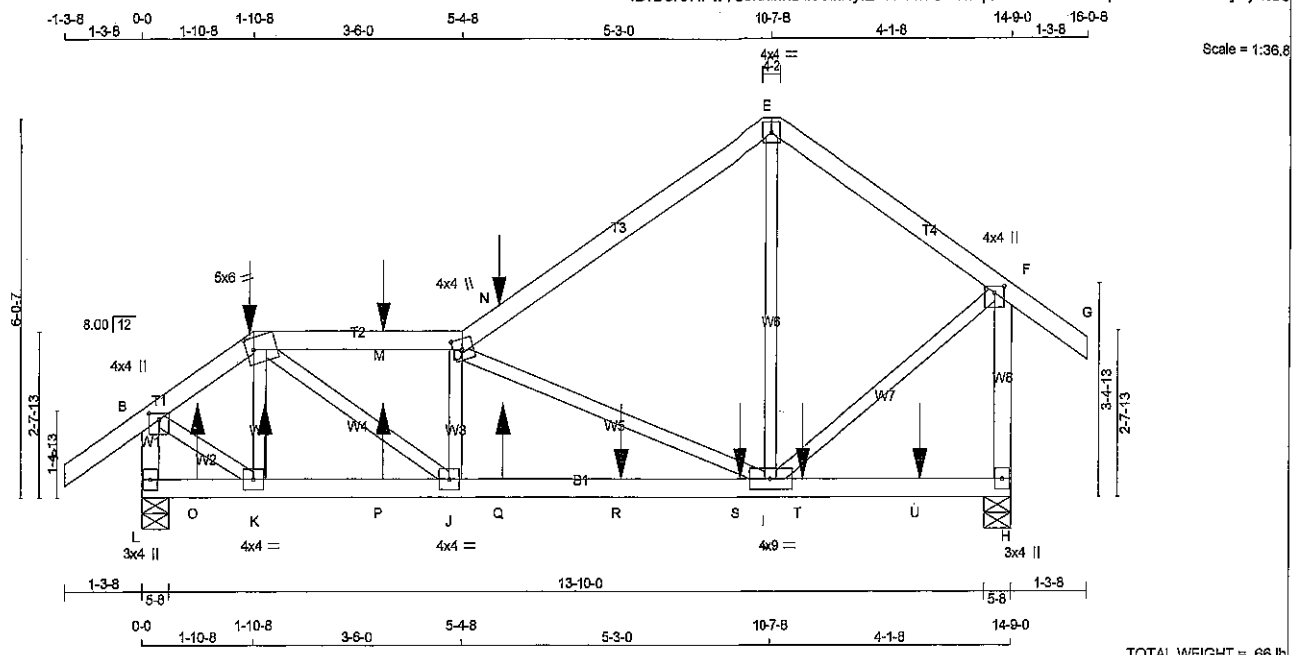

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTWW-m	MT20	5.0	6.0	2.25	1.50
D	TTWW+m	MT20	4.0	4.0	2.00	1.75
E	TTW-p	MT20	4.0	4.0		
F	TMVW+p	MT20	4.0	4.0	1.25	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMV1+p	MT20	3.0	4.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 2.4 lbs FACTORED DOWN AND 102.9 lbs FACTORED UP AT 1-10-8, AND 1.9 lbs FACTORED DOWN AND 103.4 lbs FACTORED UP AT 4-0-12, AND 1.9 lbs FACTORED DOWN AND 91.4 lbs FACTORED UP AT 6-0-12 ON TOP CHORD, AND 6.5 lbs FACTORED UP AT 11-4, 8.5 lbs FACTORED UP AT 2-0-12, 8.5 lbs FACTORED UP AT 4-0-12, 6.5 lbs FACTORED UP AT 6-0-12, 3.7 lbs FACTORED DOWN AT 8-0-12, 112.8 lbs FACTORED DOWN AT 10-0-12, AND 112.8 lbs FACTORED DOWN AT 11-2-4, AND 3.7 lbs FACTORED DOWN AT 13-2-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
L	996	0	996	0	0	5-8	5-8
H	1101	0	1101	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	795	460 / 0	158 / 0	0 / 0	0 / 0	158 / 0	0 / 0
H	664	550 / 0	156 / 0	0 / 0	0 / 0	158 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.36 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.3	-84.3	0.12 (1)	10.00	
B- C	-758 / 0	-84.3	-84.3	0.13 (1)	6.25	
C- M	-1307 / 0	-84.3	-84.3	0.22 (1)	5.36	
M- D	-1307 / 0	-84.3	-84.3	0.22 (1)	5.36	
D- N	-650 / 0	-84.3	-84.3	0.47 (1)	6.25	
N- E	-650 / 0	-84.3	-84.3	0.47 (1)	6.25	
E- F	-651 / 0	-84.3	-84.3	0.28 (1)	6.25	
F- G	0 / 32	-84.3	-84.3	0.12 (1)	10.00	
L- B	-980 / 0	0.0	0.0	0.11 (1)	7.81	
H- F	-1055 / 0	0.0	0.0	0.22 (1)	7.63	
L- O	0 / 0	-28.0	-28.0	0.04 (7)	10.00	
O- K	0 / 0	-28.0	-28.0	0.04 (7)	10.00	
K- P	0 / 597	-28.0	-28.0	0.20 (7)	10.00	
P- J	0 / 597	-28.0	-28.0	0.20 (7)	10.00	
J- Q	0 / 1324	-28.0	-28.0	0.42 (1)	10.00	
Q- R	0 / 1324	-28.0	-28.0	0.42 (1)	10.00	
R- S	0 / 1324	-28.0	-28.0	0.42 (1)	10.00	
S- I	0 / 1324	-28.0	-28.0	0.42 (1)	10.00	
I- T	0 / 0	-28.0	-28.0	0.21 (2)	10.00	
T- U	0 / 0	-28.0	-28.0	0.21 (2)	10.00	
U- H	0 / 0	-28.0	-28.0	0.21 (2)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-265 / 0	0.05 (1)
C- J	0 / 862	0.21 (1)
J- D	-380 / 0	0.07 (1)
D- I	-858 / 0	0.51 (1)
I- E	0 / 334	0.08 (2)
B- K	0 / 894	0.17 (1)
I- F	0 / 876	0.17 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	1-10-8	-1	-2	103	FRONT	VERT	TOTAL
K	2-0-12	6	—	6	FRONT	VERT	TOTAL
M	4-0-12	-1	-2	103	FRONT	VERT	TOTAL
N	6-0-12	-1	-2	91	FRONT	VERT	TOTAL
O	11-4	6	—	6	FRONT	VERT	TOTAL
P	4-0-12	6	—	6	FRONT	VERT	TOTAL
Q	6-0-12	6	—	6	FRONT	VERT	TOTAL
R	8-0-12	-4	-4	—	FRONT	VERT	TOTAL
S	10-0-12	-113	-113	—	FRONT	VERT	TOTAL
T	11-2-4	-113	-113	—	FRONT	VERT	TOTAL
U	13-2-4	-4	-4	—	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER 9117 S. KATSOULAKOS PROVINCE OF ONTARIO

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.49")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.49")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.47 (D-E:1) , BC=0.42 (I-J:1) , WB=0.51 (D-I:1) , SSI=0.17 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (J) (INPUT = 0.90)
JSI METAL= 0.29 (D) (INPUT = 1.00)

DWG NO. TAM45597-17
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285956	T123	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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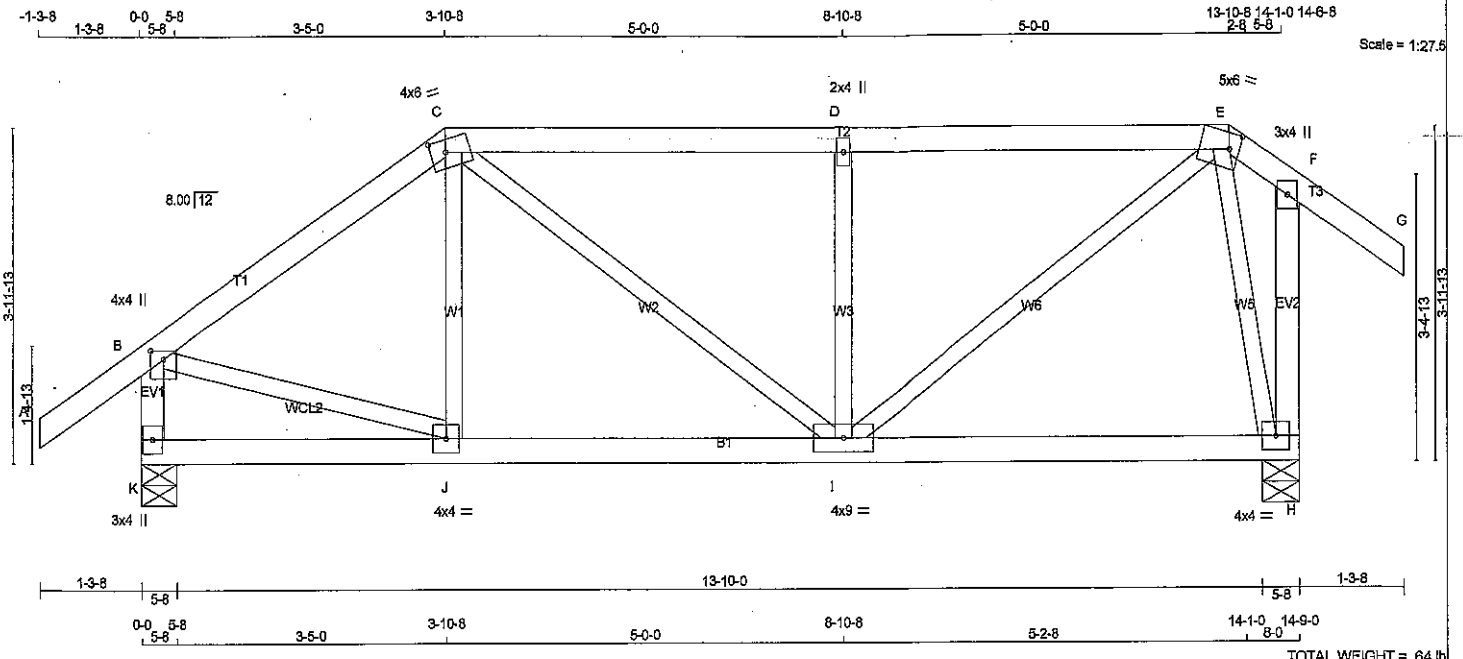

TOTAL WEIGHT = 64 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER		DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTWW-m	MT20	4.0	6.0	1.75	2.25
D	TMW+w	MT20	2.0	4.0		
E	TTWW-m	MT20	5.0	6.0	2.25	1.50
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS 14-9-0

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	938	0	938	0	0	5-8	5-8
H	949	0	949	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	755	444 / 0	155 / 0	0 / 0	0 / 0	155 / 0	0 / 0
H	762	451 / 0	155 / 0	0 / 0	0 / 0	156 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.04 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	J-C	-50 / 127	0.03 (3)
B-C	-785 / 0	-84.3	-84.3	0.23 (1)	8.25	C-I	0 / 271	0.06 (1)
C-D	-869 / 0	-84.3	-84.3	0.36 (1)	6.04	I-D	-516 / 0	0.13 (1)
D-E	-869 / 0	-84.3	-84.3	0.36 (1)	6.04	B-J	0 / 676	0.15 (1)
E-F	-51 / 0	-84.3	-84.3	0.11 (1)	6.25	E-H	-655 / 0	0.17 (1)
F-G	0 / 32	-84.3	-84.3	0.11 (1)	10.00	I-E	0 / 891	0.20 (1)
K-B	-893 / 0	0.0	0.0	0.09 (1)	7.81			
H-F	-243 / 0	0.0	0.0	0.05 (1)	7.81			
K-J	0 / 0	-28.0	-28.0	0.10 (3)	10.00			
J-I	0 / 651	-28.0	-28.0	0.29 (2)	10.00			
I-H	0 / 152	-28.0	-28.0	0.23 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.49")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.49")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.36/1.00 (D-E:1) , BC=0.29/1.00 (I-J:2) , WB=0.20/1.00 (E-I:1) , SSI=0.20/1.00 (C-D:1)

OOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (E) (INPUT = 0.90)
JSI METAL= 0.24 (J) (INPUT = 1.00)

DWG NO. TAM 26885-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285956	T124A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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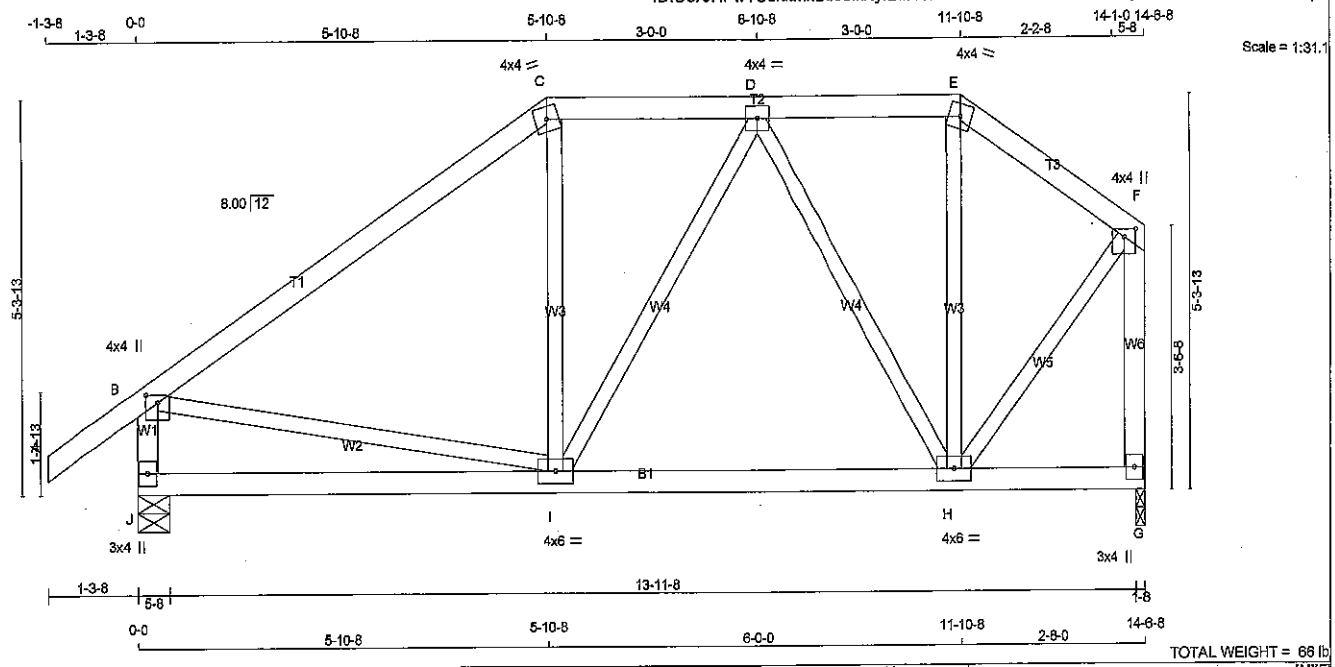

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

ORY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTW-m	MT20	4.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TTW-m	MT20	4.0	4.0		
F	TMVW+p	MT20	4.0	4.0	1.25	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	6.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	932	0	932	0	0	5-8	5-8
G	816	0	816	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	749	442 / 0	153 / 0	0 / 0	0 / 0	154 / 0	0 / 0
G	670	372 / 0	153 / 0	0 / 0	0 / 0	145 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00
B- C	-711 / 0	-84.3	-84.3	0.38 (1)	8.25
C- D	-583 / 0	-84.3	-84.3	0.10 (1)	8.25
D- E	-372 / 0	-84.3	-84.3	0.09 (1)	8.25
E- F	-448 / 0	-84.3	-84.3	0.08 (1)	6.25
J- B	-684 / 0	0.0	0.0	0.09 (1)	7.81
G- F	-794 / 0	0.0	0.0	0.17 (1)	7.81
J- I	0 / 0	-28.0	-28.0	0.26 (3)	10.00
I- H	0 / 571	-28.0	-28.0	0.32 (2)	10.00
H- G	0 / 0	-28.0	-28.0	0.12 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- C	0 / 181	0.04 (3)
I- D	0 / 114	0.03 (3)
D- H	-398 / 0	0.23 (1)
H- E	0 / 113	0.03 (3)
B- I	0 / 800	0.14 (1)
H- F	0 / 570	0.13 (1)

[M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.8	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.48")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.48")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.38/1.00 (B-C:1) , BC=0.32/1.00 (H-I:2) , WB=0.23/1.00 (D-H:1) , SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (I) (INPUT = 0.90)
JSI METAL= 0.21 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEERS
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 5111 -R8
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285956	T125A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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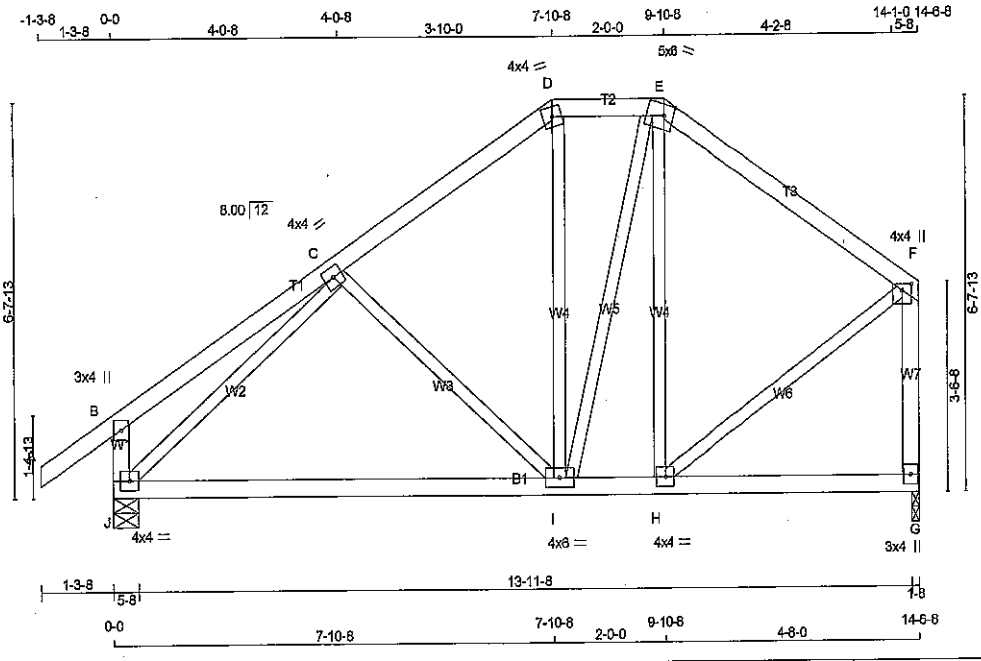

TOTAL WEIGHT = 71 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW-m	MT20	4.0	4.0		
E	TTWW-m	MT20	5.0	6.0	2.25	2.25
F	TMVW+p	MT20	4.0	4.0	1.25	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	816	0	816	0	0	1-8	1-8
J	932	0	932	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	870	372 / 0	153 / 0	0 / 0	0 / 0	145 / 0	0 / 0
J	749	442 / 0	153 / 0	0 / 0	0 / 0	154 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	C- I	-202 / 30	0.10 (1)
B- C	0 / 21	-84.3	-84.3	0.21 (1)	10.00	I- D	0 / 173	0.04 (2)
C- D	-607 / 0	-84.3	-84.3	0.16 (1)	6.25	I- E	0 / 276	0.06 (2)
D- E	-489 / 0	-84.3	-84.3	0.05 (1)	6.25	H- E	-252 / 0	0.18 (1)
E- F	-503 / 0	-84.3	-84.3	0.24 (1)	6.25	J- C	-874 / 0	0.41 (1)
J- B	-242 / 0	0.0	0.0	0.02 (1)	7.81	H- F	0 / 504	0.11 (1)
G- F	-758 / 0	0.0	0.0	0.16 (1)	7.81			
J- I	0 / 636	-26.0	-26.0	0.48 (2)	10.00			
I- H	0 / 413	-26.0	-26.0	0.47 (2)	10.00			
H- G	0 / 0	-26.0	-26.0	0.15 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.48")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.48")
CALCULATED VERT. DEFL.(TL) = L/ 722 (0.24")

CSI: TC=0.24/1.00 (E-F:1) , BC=0.48/1.00 (I-J:2) , WB=0.41/1.00 (C-J:1) , SSI=0.16/1.00 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (J) (INPUT = 0.90)
JSI METAL= 0.30 (C) (INPUT = 1.00)

DWG NO. TAM 5112 -1R
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285952	T127	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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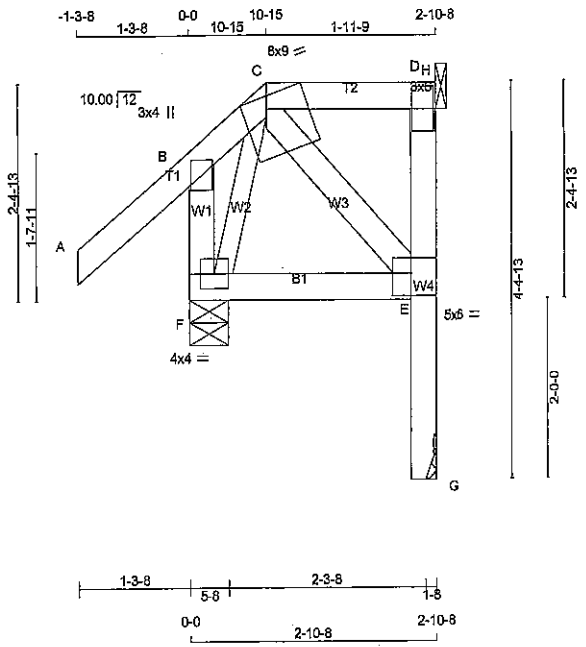

Scale = 1:25.6

TOTAL WEIGHT = 2 X 19 = 38 lb

[M

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - H	2x4	DRY	No.2	SPF
G - D	2x4	DRY	No.2	SPF
F - B	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
F - C	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TTWW-m	MT20	8.0	9.0	Edge	3.25
D	TMV1+t	MT20	3.0	6.0		
E	BVMW-l	MT20	5.0	6.0	3.00	2.50
F	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
D	116	0	116	0	0	1-8	1-8
G	25	0	38	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
F	301	0	301	0	0	5-8	5-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) D

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	90	59 / -9	15 / 0	0 / 0	0 / 0	16 / 0	0 / 0
G	29	2 / -8	16 / 0	0 / 0	0 / 0	11 / 0	0 / 0
F	227	159 / 0	30 / 0	0 / 0	0 / 0	38 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 37	-84.3	-84.3	0.12 (1)	10.00	C- E	-13 / 35	0.01 (5)
B- C	-56 / 0	-84.3	-84.3	0.11 (1)	6.25	F- C	-19 / 60	0.01 (5)
C- D	0 / 2	-84.3	-84.3	0.05 (1)	10.00			
D- H	0 / 0	-76.8	-76.8	0.00 (1)	10.00			
G- E	-36 / 0	0.0	0.0	0.01 (2)	7.81			
E- D	-1 / 37	0.0	0.0	0.01 (2)	7.81			
F- B	-244 / 0	0.0	0.0	0.03 (1)	7.81			
F- E	-24 / 7	-28.0	-28.0	0.07 (3)	6.25			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.12 (A-B:1) , BC=0.07 (E-F:3) , WB=0.01 (C-F:5) , SSI=0.08 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2264	1556

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (F) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)

DWG NO. TAM 4560-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285952	T128	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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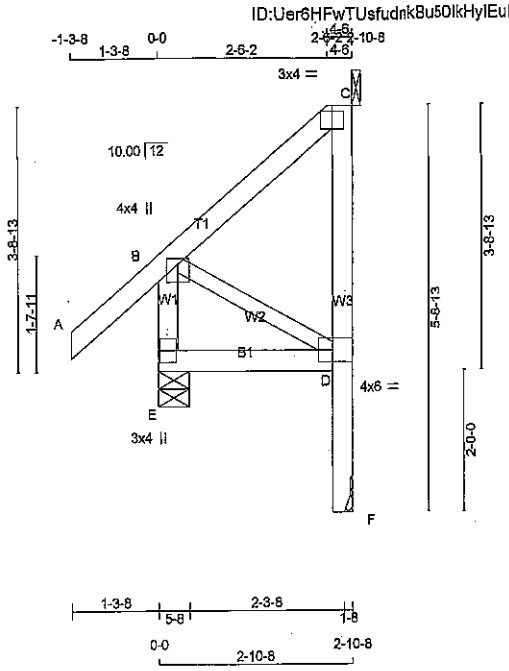

Scale = 1:32.6

TOTAL WEIGHT = 2 X 19 = 38 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
F - C	2x4	DRY	No.2	SPF
E - B	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TVM1-p	MT20	3.0	4.0	Edge	2.00
D	BVMW-I	MT20	4.0	6.0	2.00	2.50
E	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
C	136	0	136	0	0	1-8	1-8
F	25	0	45	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
E	278	0	278	0	0	5-8	5-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	101	74 / 0	11 / 0	0 / 0	0 / 0	16 / 0	0 / 0
F	32	0 / 0	19 / 0	0 / 0	0 / 0	13 / 0	0 / 0
E	212	144 / 0	30 / 0	0 / 0	0 / 0	37 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 37	-84.3	-84.3	0.12 (1)	10.00	B- D	0 / 0	0.00 (1)
B- C	0 / 0	-84.3	-84.3	0.12 (1)	10.00			
F- D	-45 / 0	0.0	0.0	0.01 (3)	7.81			
D- C	0 / 26	0.0	0.0	0.00 (3)	10.00			
E- B	-238 / 0	0.0	0.0	0.03 (1)	7.81			
E- D	0 / 0	-26.0	-26.0	0.07 (3)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.12 (B-C:1) , BC=0.07 (D-E:3) , WB=0.00 (B-D:1) , SSI=0.07 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (B) (INPUT = 0.90)
JSI METAL= 0.06 (C) (INPUT = 1.00)

DWG NO. TAM45602-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285952	T129	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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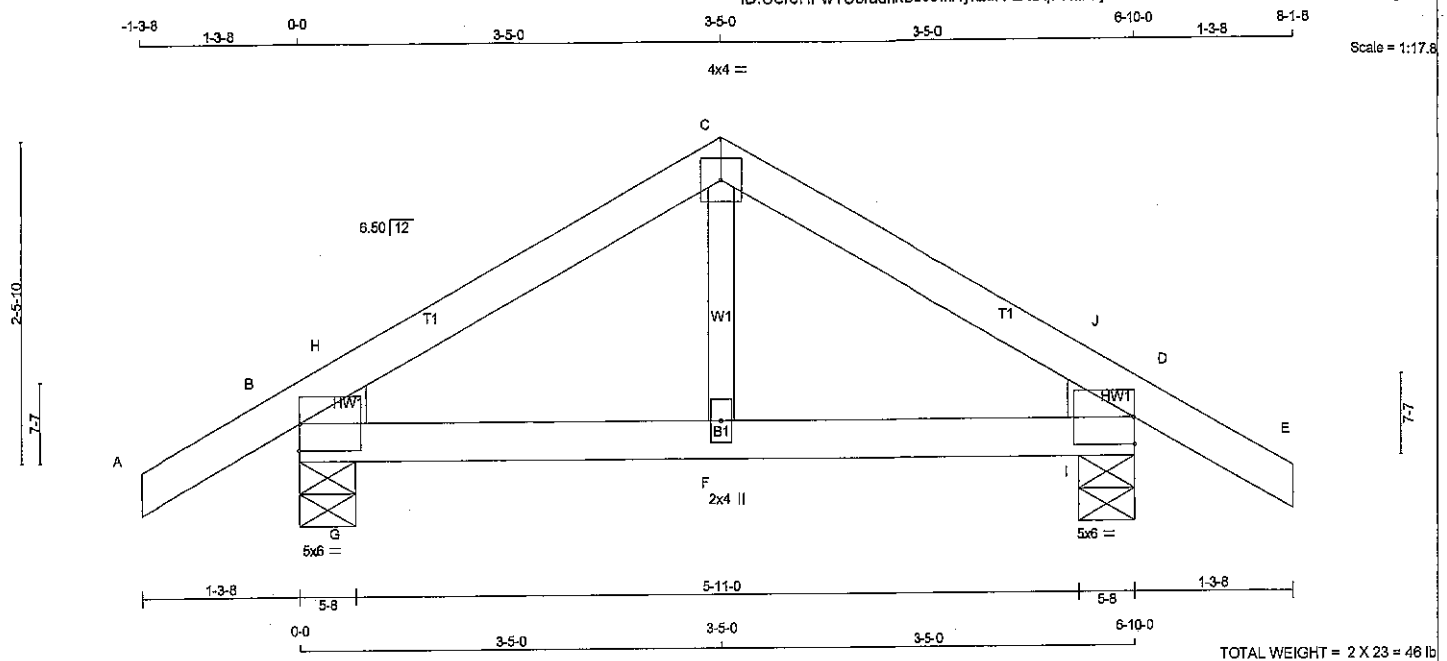


TOTAL WEIGHT = 2 X 23 = 46 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMBH1-I	MT20	5.0	6.0	
C	TTW-p	MT20	4.0	4.0	
D	TMBH1-I	MT20	5.0	6.0	
F	BMW+w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG	HEEL
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	WEDGE
B	498	0	498	0	0	5-8	5-8	2x4 L
D	498	0	498	0	0	5-8	5-8	2x4 R

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	393	244 / 0	72 / 0	0 / 0	0 / 0	77 / 0	0 / 0
D	393	244 / 0	72 / 0	0 / 0	0 / 0	77 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 17	-84.3	-84.3	0.11 (1)	10.00	F- C	0 / 176	0.04 (2)
B- H	-431 / 0	-84.3	-84.3	0.06 (1)	6.25	G- H	-42 / 121	0.00 (1)
H- C	-377 / 0	-84.3	-84.3	0.09 (1)	6.25	I- J	-42 / 121	0.00 (1)
C- J	-377 / 0	-84.3	-84.3	0.09 (1)	6.25			
J- D	-431 / 0	-84.3	-84.3	0.06 (1)	6.25			
D- E	0 / 17	-84.3	-84.3	0.11 (1)	10.00			
B- G	0 / 324	-28.0	-28.0	0.09 (1)	10.00			
G- F	0 / 324	-28.0	-28.0	0.11 (1)	10.00			
F- I	0 / 324	-28.0	-28.0	0.11 (1)	10.00			
I- D	0 / 324	-28.0	-28.0	0.09 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.23")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.11 (D-E:1) , BC=0.11 (F-G:1) , WB=0.04 (C-F:2) , SSI=0.10 (C-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

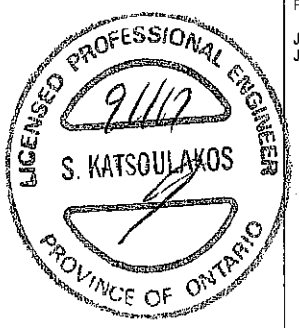
NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1867	822	2284	1655

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (D) (INPUT = 0.90)
JSI METAL= 0.11 (D) (INPUT = 1.00)



DWG NO. TAM 456031 17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285952	T129Z	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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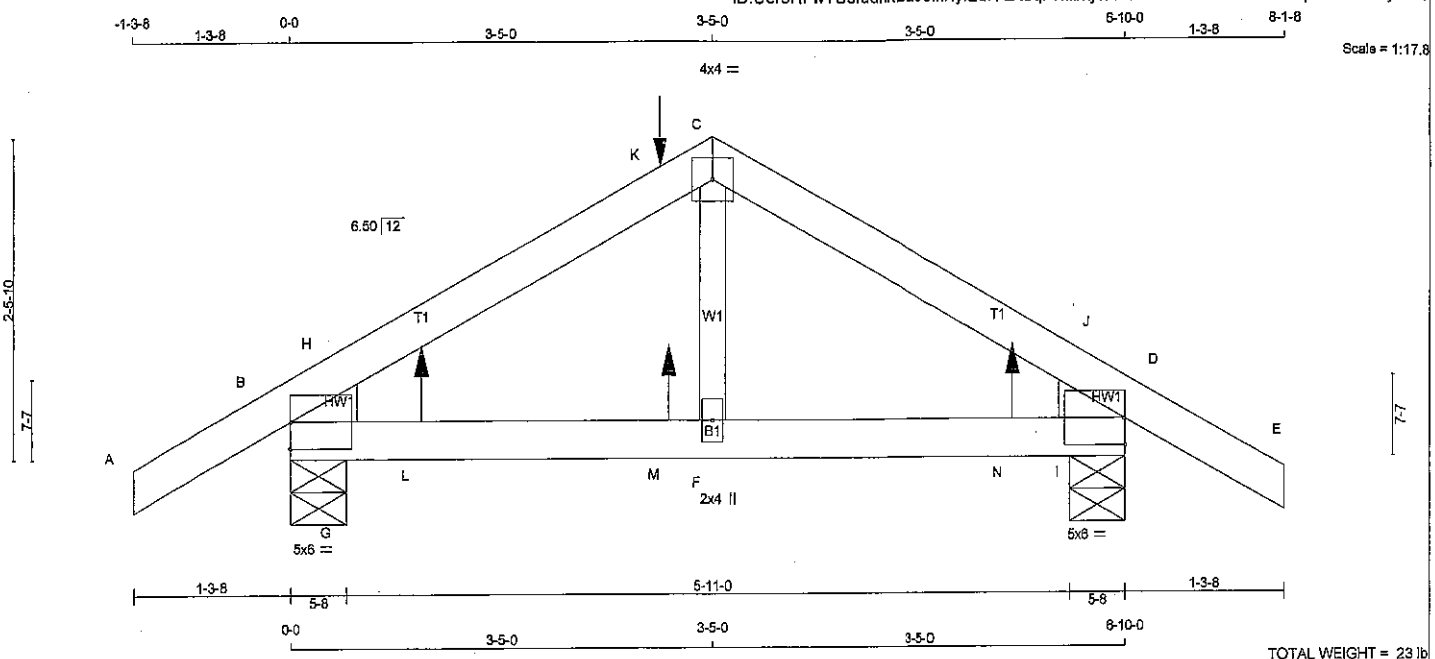

TOTAL WEIGHT = 23 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMBH1-I	MT20	5.0	6.0	
C	TTW-p	MT20	4.0	4.0	
D	TMBH1-I	MT20	5.0	6.0	
F	BMW+w	MT20	2.0	4.0	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1.9 lbs FACTORED DOWN AND 78.6 lbs FACTORED UP AT 3-0-12 ON TOP CHORD, AND 15.9 lbs FACTORED UP AT 1-0-12, AND 15.9 lbs FACTORED UP AT 3-0-12, AND 15.9 lbs FACTORED UP AT 5-10-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG	HEEL
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	WEDGE
B	483	0	483	0	0	5-8	5-8	2x4 L
D	484	0	484	0	0	5-8	5-8	2x4 R

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	383	234 / 0	72 / 0	0 / 0	0 / 0	77 / 0	0 / 0
D	384	235 / 0	72 / 0	0 / 0	0 / 0	77 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS						WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO				FR-TO		
A- B	0 / 17	-84.3 -84.3	0.12 (1)	10.00		F- C	0 / 171	0.05 (7)
B- H	-408 / 0	-84.3 -84.3	0.08 (1)	6.25		G- H	-67 / 121	0.00 (1)
H- K	-382 / 0	-84.3 -84.3	0.11 (1)	6.25		I- J	-84 / 121	0.00 (1)
K- C	-362 / 0	-84.3 -84.3	0.11 (1)	6.25				
C- J	-362 / 0	-84.3 -84.3	0.11 (5)	6.25				
J- D	-410 / 0	-84.3 -84.3	0.08 (1)	6.25				
D- E	0 / 17	-84.3 -84.3	0.12 (1)	10.00				
B- G	0 / 311	-28.0 -28.0	0.10 (1)	10.00				
G- L	0 / 311	-28.0 -28.0	0.12 (1)	10.00				
L- M	0 / 311	-28.0 -28.0	0.12 (1)	10.00				
M- F	0 / 311	-28.0 -28.0	0.12 (1)	10.00				
F- N	0 / 311	-28.0 -28.0	0.12 (1)	10.00				
N- I	0 / 311	-28.0 -28.0	0.12 (1)	10.00				
I- D	0 / 311	-28.0 -28.0	0.10 (5)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
K	3-0-12	-1	-2	79	FRONT	VERT	TOTAL
L	1-0-12	10	—	16	FRONT	VERT	TOTAL
M	3-0-12	10	—	16	FRONT	VERT	TOTAL
N	5-10-12	10	—	16	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.23")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.12 (D-E:1) , BC=0.12 (F-G:1) , WB=0.05 (C-F:7) , SSI=0.11 (C-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (D) (INPUT = 0.90)
JSI METAL= 0.10 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
9/11/17
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 45604-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285956	T203A	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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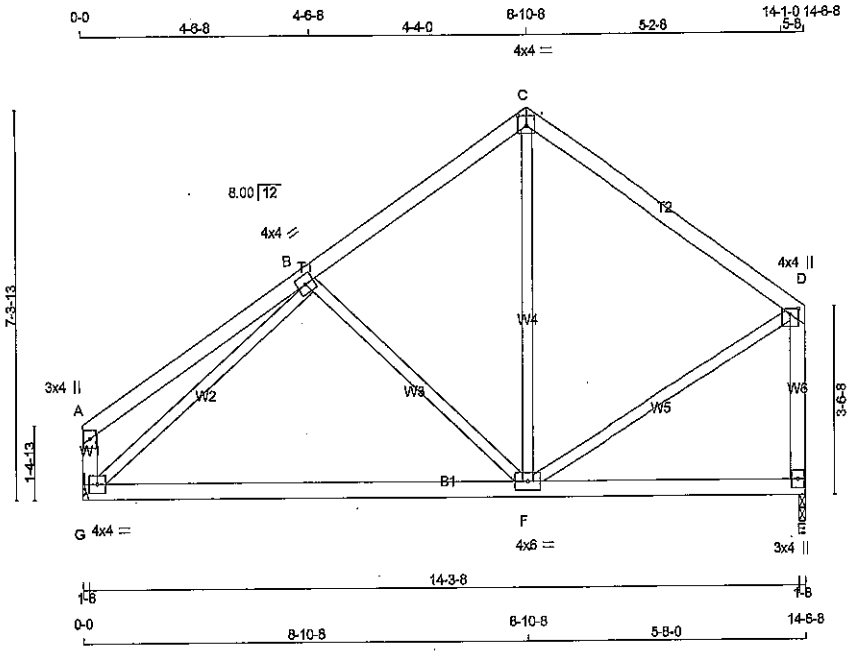

TOTAL WEIGHT = 3 X 61 = 184 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
G - A	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	4.0		
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMVW+p	MT20	4.0	4.0	1.25	2.00
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	6.0		
G	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	616	0	616	0	0	1-8	1-8
G	616	0	616	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	670	372 / 0	153 / 0	0 / 0	0 / 0	145 / 0	0 / 0
G	670	372 / 0	153 / 0	0 / 0	0 / 0	145 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 25	-84.3	-84.3	0.27 (1)	10.00
B- C	-545 / 0	-84.3	-84.3	0.21 (1)	6.25
C- D	-520 / 0	-84.3	-84.3	0.35 (1)	6.25
G- A	-142 / 0	0.0	0.0	0.01 (1)	7.81
E- D	-773 / 0	0.0	0.0	0.16 (1)	7.81
G- F	0 / 629	-28.0	-28.0	0.58 (2)	10.00
F- E	0 / 0	-28.0	-28.0	0.50 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- F	-265 / 17	0.17 (1)
F- C	0 / 312	0.07 (2)
G- B	-851 / 0	0.52 (1)
F- D	0 / 498	0.11 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.48")
CALCULATED VERT. DEFL.(LL) = L/ 775 (0.23")
ALLOWABLE DEFL.(TL)= L/360 (0.48")
CALCULATED VERT. DEFL.(TL) = L/ 466 (0.37")

CSI: TC=0.35/1.00 (C-D:1) , BC=0.56/1.00 (F-G:2) , WB=0.52/1.00 (B-G:1) , SSI=0.20/1.00 (F-G:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.72 (G) (INPUT = 0.90)
JSI METAL= 0.29 (B) (INPUT = 1.00)

DWG NO. TAM 5113 -R8
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285952	T205	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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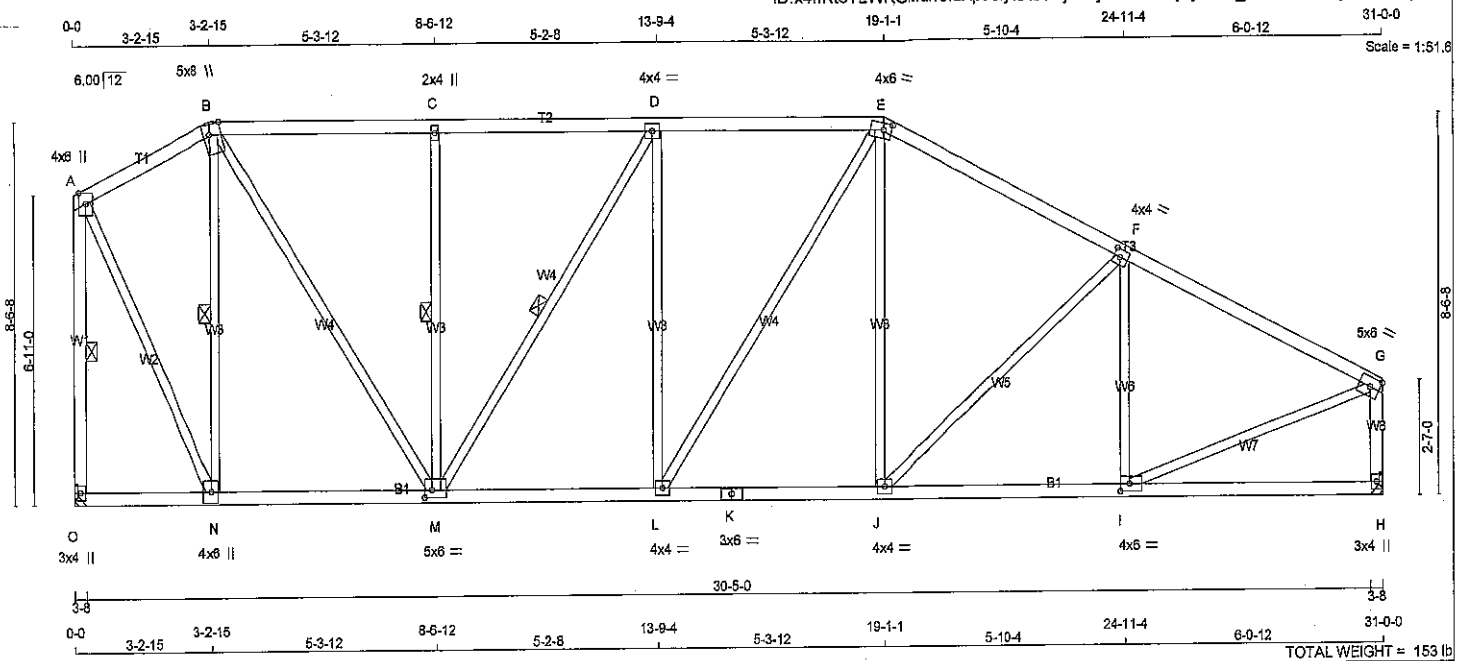

TOTAL WEIGHT = 153 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
O - A	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
B	TTWW+m	MT20	5.0	6.0	Edge	3.50
C	TMW+w	MT20	2.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TTWW-m	MT20	4.0	6.0	1.75	2.25
F	TMWW-t	MT20	4.0	4.0	2.00	1.75
G	TMVW-t	MT20	5.0	6.0		Edge
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	6.0	2.00	2.50
J	BMWW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMWWW-t	MT20	5.0	6.0	2.00	2.00
N	BMWW+t	MT20	4.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1740	0	1740	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	
H	1740	0	1740	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1429	793 / 0	325 / 0	0 / 0	0 / 0	310 / 0	0 / 0
H	1429	793 / 0	325 / 0	0 / 0	0 / 0	310 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.17 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF B-N, C-M, D-M, A-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-711 / 0	-84.3	-84.3	0.16 (1)	6.25
B- C	-1341 / 0	-84.3	-84.3	0.35 (1)	5.17
C- D	-1341 / 0	-84.3	-84.3	0.36 (1)	5.15
D- E	-1649 / 0	-84.3	-84.3	0.38 (1)	4.73
E- F	-1766 / 0	-84.3	-84.3	0.83 (1)	4.27
F- G	-1857 / 0	-84.3	-84.3	0.65 (1)	4.17
O- A	-1710 / 0	0.0	0.0	0.38 (1)	5.09
H- G	-1671 / 0	0.0	0.0	0.22 (1)	6.42
O- N	0 / 0	-28.0	-28.0	0.13 (2)	10.00
N- M	0 / 621	-28.0	-28.0	0.22 (2)	10.00
M- L	0 / 1549	-28.0	-28.0	0.34 (1)	10.00
L- K	0 / 1558	-28.0	-28.0	0.34 (2)	10.00
K- J	0 / 1558	-28.0	-28.0	0.34 (2)	10.00
J- I	0 / 1685	-28.0	-28.0	0.44 (2)	10.00
I- H	0 / 0	-28.0	-28.0	0.25 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- B	-1117 / 0	0.52 (1)
B- M	0 / 1327	0.30 (1)
M- C	-482 / 0	0.22 (1)
M- D	-578 / 0	0.39 (1)
L- D	0 / 202	0.05 (3)
L- E	0 / 169	0.04 (1)
J- E	0 / 341	0.08 (2)
J- F	-175 / 0	0.23 (1)
I- F	-443 / 19	0.21 (1)
A- N	0 / 1405	0.32 (1)
I- G	0 / 1800	0.41 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.65 (F-G:1) , BC=0.44 (I-J:2) , WB=0.52 (B-N:1) , SSI=0.22 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.48 (G) (INPUT = 1.00)

DWG NO. TAM456064.17
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285952	T206	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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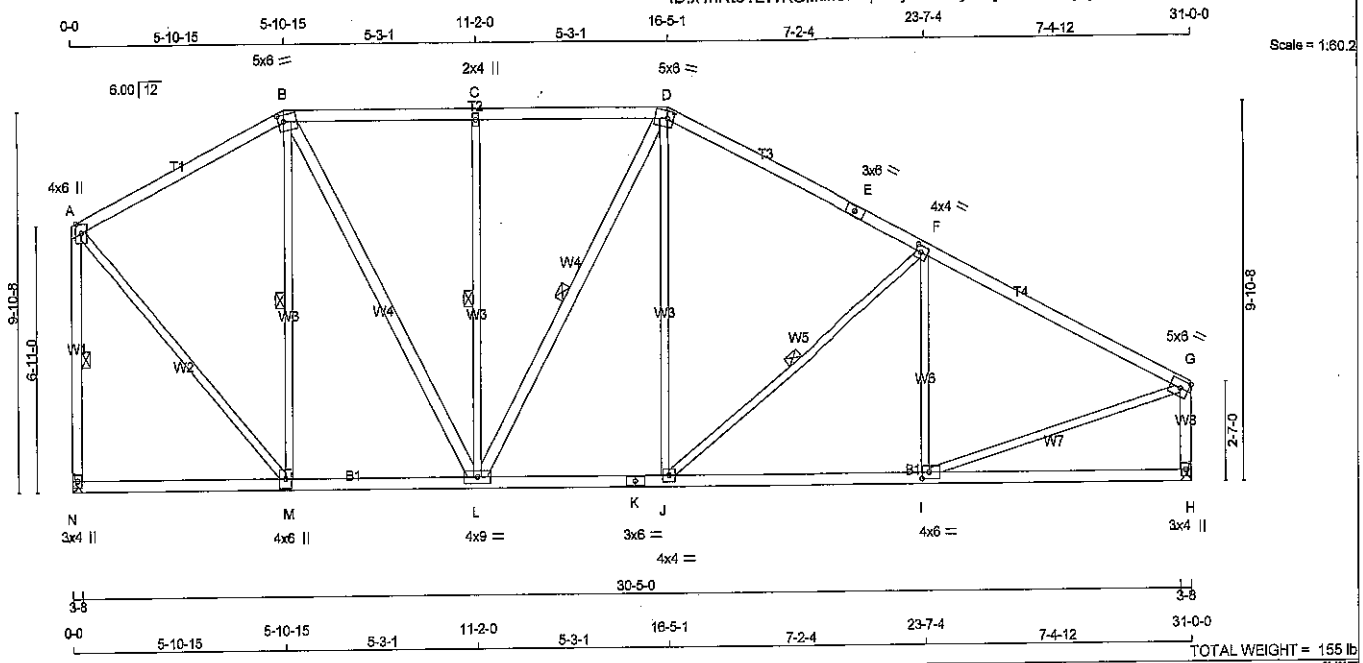

TOTAL WEIGHT = 155 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
N - A	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
B - L	2x4	DRY	No.2	SPF
L - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW+p	MT20	4.0	6.0	Edge	
B	TTWW-m	MT20	5.0	6.0	2.00	1.75
C	TMW+w	MT20	2.0	4.0		
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0	2.00	1.75
G	TMW-t	MT20	5.0	6.0		Edge
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	6.0	2.00	2.25
J	BMWW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	4.0	9.0		
M	BMWW+t	MT20	4.0	6.0		
N	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1740	0	1740	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	
H	1740	0	1740	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1429	793 / 0	325 / 0	0 / 0	0 / 0	310 / 0	0 / 0
H	1429	793 / 0	325 / 0	0 / 0	0 / 0	310 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF B-M, C-L, D-L, F-J, A-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	-995 / 0	-84.3	-84.3	0.40 (1)		5.74
B- C	-1330 / 0	-84.3	-84.3	0.32 (1)		5.26
C- D	-1330 / 0	-84.3	-84.3	0.32 (1)		5.26
D- E	-1610 / 0	-84.3	-84.3	0.67 (1)		4.36
E- F	-1610 / 0	-84.3	-84.3	0.67 (1)		4.36
F- G	-1932 / 0	-84.3	-84.3	0.72 (1)		3.98
N- A	-1672 / 0	0.0	0.0	0.37 (1)		5.13
H- G	-1657 / 0	0.0	0.0	0.22 (1)		6.44
N- M	0 / 0	-28.0	-28.0	0.24 (2)		10.00
M- L	0 / 881	-28.0	-28.0	0.34 (2)		10.00
L- K	0 / 1414	-28.0	-28.0	0.38 (2)		10.00
K- J	0 / 1414	-28.0	-28.0	0.38 (2)		10.00
J- I	0 / 1758	-28.0	-28.0	0.59 (2)		10.00
I- H	0 / 0	-28.0	-28.0	0.39 (3)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- B	-792 / 0	0.52 (1)
B- L	0 / 933	0.15 (1)
L- C	-544 / 0	0.36 (1)
L- D	-174 / 0	0.11 (1)
J- D	0 / 502	0.11 (2)
J- F	-451 / 0	0.28 (1)
I- F	-306 / 145	0.19 (1)
A- M	0 / 1322	0.30 (1)
I- G	0 / 1839	0.41 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.72 (F-G:1) , BC=0.59 (I-J:2) , WB=0.52 (B-M:1) , SSI=0.27 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (I) (INPUT = 0.90)
JSI METAL= 0.51 (G) (INPUT = 1.00)

DWG NO. TAM45007-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285952	T207	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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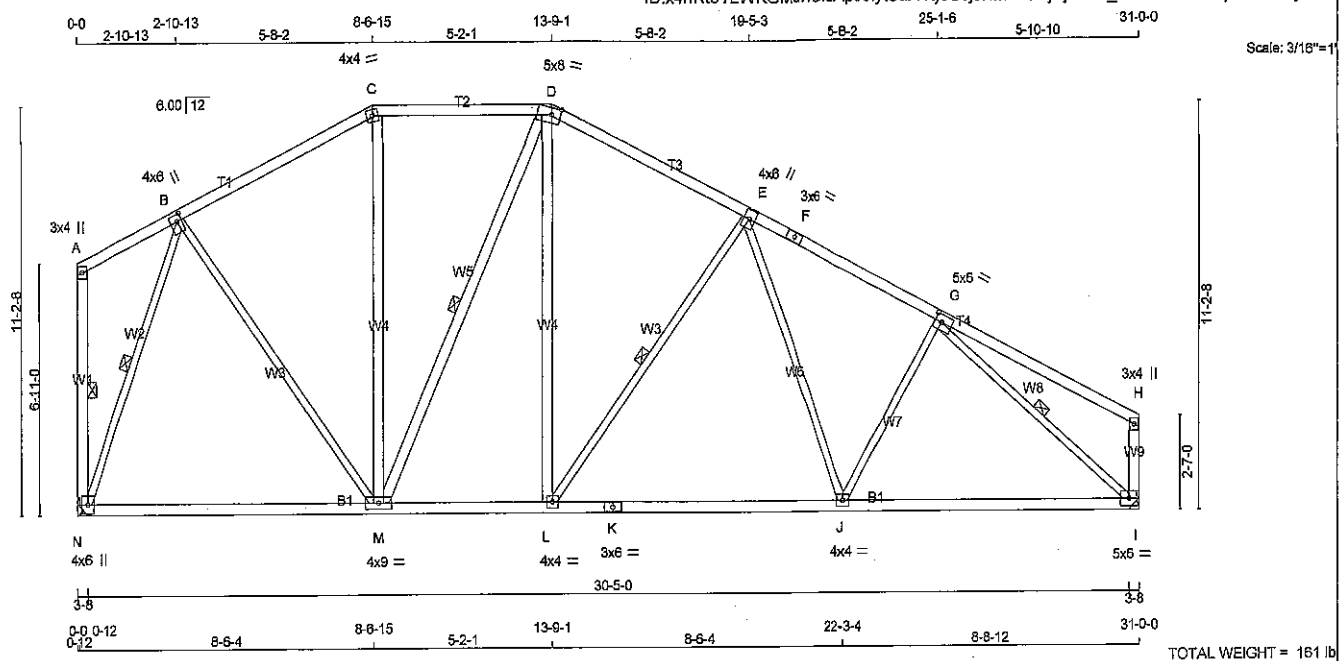


TOTAL WEIGHT = 161 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
N - A	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
M - C	2x4	DRY	No.2	SPF
M - D	2x4	DRY	No.2	SPF
L - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW+t	MT20	4.0	6.0	2.25	1.75
C	TTW-m	MT20	4.0	4.0		
D	TTWW-m	MT20	5.0	8.0	2.25	3.00
E	TMWW+t	MT20	4.0	6.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	5.0	6.0	2.50	2.25
H	TMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	5.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMWWW-t	MT20	4.0	9.0		
N	BMVW1+p	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	1740	0	1740	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	
N	1740	0	1740	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1429	793 / 0	325 / 0	0 / 0	0 / 0	310 / 0	0 / 0
N	1429	793 / 0	325 / 0	0 / 0	0 / 0	310 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.59 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-M, E-L, A-N, G-I, B-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 37	-84.3	-84.3	0.33 (1)	10.00	B- M	0 / 747	0.17 (1)
B- C	-1152 / 0	-84.3	-84.3	0.34 (1)	5.53	M- C	0 / 228	0.04 (3)
C- D	-1014 / 0	-84.3	-84.3	0.30 (1)	5.84	M- D	-514 / 0	0.42 (1)
D- E	-1386 / 0	-84.3	-84.3	0.35 (1)	5.15	L- D	0 / 868	0.11 (1)
E- F	-1862 / 0	-84.3	-84.3	0.35 (1)	4.59	L- E	-636 / 0	0.45 (1)
F- G	-1862 / 0	-84.3	-84.3	0.35 (1)	4.59	E- J	0 / 332	0.07 (2)
G- H	0 / 21	-84.3	-84.3	0.37 (1)	10.00	J- G	0 / 232	0.05 (3)
N- A	-38 / 0	0.0	0.0	0.01 (1)	6.25	G- I	-2171 / 0	0.86 (1)
I- H	-198 / 0	0.0	0.0	0.03 (1)	7.81	N- B	-1700 / 0	0.84 (1)
N- M	0 / 574	-28.0	-28.0	0.50 (2)	10.00			
M- L	0 / 1229	-28.0	-28.0	0.58 (2)	10.00			
L- K	0 / 1594	-28.0	-28.0	0.75 (2)	10.00			
K- J	0 / 1594	-28.0	-28.0	0.75 (2)	10.00			
J- I	0 / 1619	-28.0	-28.0	0.75 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.24")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 938 (0.40")

CSI: TC=0.37 (G-H:1) , BC=0.75 (I-J:2) , WB=0.86 (G-I:1) , SSI=0.20 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

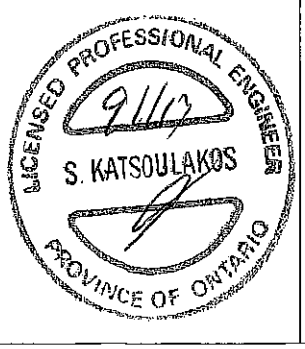
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.54 (K) (INPUT = 1.00)



DWG NO. TAM45609-17

STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285955	T208A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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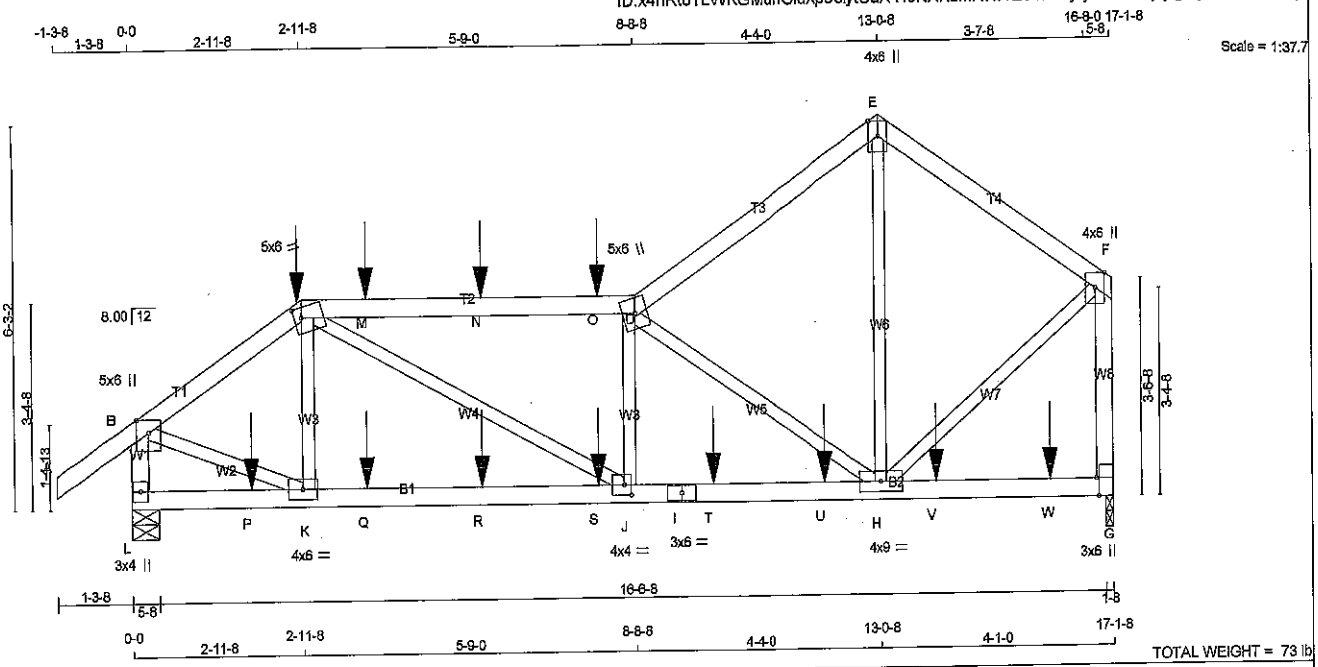


TOTAL WEIGHT = 73 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW-m	MT20	5.0	6.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0		
E	TTW+p	MT20	4.0	6.0	Edge	
F	TMVW+p	MT20	4.0	6.0	Edge	
G	BMV1+p	MT20	3.0	6.0	Edge	0.50
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0	2.00	1.50
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 76.5 lbs FACTORED DOWN AT 2-11-8, 9.4 lbs FACTORED DOWN AT 4-0-12, AND 9.4 lbs FACTORED DOWN AT 6-0-12, AND 24.4 lbs FACTORED DOWN AT 8-0-12 ON TOP CHORD, AND 14.8 lbs FACTORED DOWN AT 2-0-12, 14.8 lbs FACTORED DOWN AT 4-0-12, 14.8 lbs FACTORED DOWN AT 6-0-12, 14.8 lbs FACTORED DOWN AT 8-0-12, 173.8 lbs FACTORED DOWN AT 10-0-12, 194.2 lbs FACTORED DOWN AT 12-0-12, AND 194.2 lbs FACTORED DOWN AT 14-0-4, AND 173.8 lbs FACTORED DOWN AT 16-0-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1368	0	1368	0	0	5-8	5-8
G	1562	0	1562	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1092	653 / 0	217 / 0	0 / 0	0 / 0	222 / 0	0 / 0
G	1250	746 / 0	250 / 0	0 / 0	0 / 0	254 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.85 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.3	-84.3	0.12 (1)	10.00
B- C	-1291 / 0	-84.3	-84.3	0.16 (1)	5.47
C- M	-1970 / 0	-84.3	-84.3	0.71 (1)	3.85
M- N	-1970 / 0	-84.3	-84.3	0.71 (1)	3.85
N- O	-1970 / 0	-84.3	-84.3	0.71 (1)	3.85
O- D	-1970 / 0	-84.3	-84.3	0.71 (1)	3.85
D- E	-1008 / 0	-84.3	-84.3	0.33 (1)	5.76
E- F	-1010 / 0	-84.3	-84.3	0.29 (1)	5.82
L- B	-1338 / 0	0.0	0.0	0.15 (1)	6.97
G- F	-1394 / 0	0.0	0.0	0.31 (1)	6.85
L- P	0 / 0	-28.0	-28.0	0.18 (3)	10.00
P- K	0 / 0	-28.0	-28.0	0.18 (3)	10.00
K- Q	0 / 1064	-28.0	-28.0	0.34 (2)	10.00
Q- R	0 / 1064	-28.0	-28.0	0.34 (2)	10.00
R- S	0 / 1064	-28.0	-28.0	0.34 (2)	10.00
S- J	0 / 1064	-28.0	-28.0	0.34 (2)	10.00
J- I	0 / 1977	-28.0	-28.0	0.73 (1)	10.00
I- T	0 / 1977	-28.0	-28.0	0.73 (1)	10.00
T- U	0 / 1977	-28.0	-28.0	0.73 (1)	10.00
U- H	0 / 1977	-28.0	-28.0	0.73 (1)	10.00
H- V	0 / 0	-28.0	-28.0	0.39 (1)	10.00
V- W	0 / 0	-28.0	-28.0	0.39 (1)	10.00
W- G	0 / 0	-28.0	-28.0	0.39 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-230 / 92	0.05 (1)
C- J	0 / 1030	0.26 (1)
J- D	-208 / 99	0.04 (1)
D- H	-1392 / 0	0.68 (1)
H- E	0 / 763	0.19 (1)
B- K	0 / 1137	0.28 (1)
H- F	0 / 1070	0.26 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
C	2-11-8	-8	-8	---	FRONT	VERT	DEAD
C	2-11-8	-69	-69	---	FRONT	VERT	SNOW
M	4-0-12	-9	-9	---	BACK	VERT	TOTAL
N	6-0-12	-9	-9	---	BACK	VERT	TOTAL
O	8-0-12	-24	-24	---	BACK	VERT	TOTAL
P	2-0-12	-8	-15	---	BACK	VERT	TOTAL
Q	4-0-12	-8	-15	---	BACK	VERT	TOTAL
R	6-0-12	-8	-15	---	BACK	VERT	TOTAL
S	8-0-12	-8	-15	---	BACK	VERT	TOTAL
T	10-0-12	-174	-174	---	BACK	VERT	TOTAL
U	12-0-12	-194	-194	---	BACK	VERT	TOTAL
V	14-0-4	-194	-194	---	BACK	VERT	TOTAL

[M]

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.8	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.57")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.57")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.71/1.00 (C-D:1) , BC=0.73/1.00 (H-J:1) , WB=0.68/1.00 (D-H:1) , SSI=0.27/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (C) (INPUT = 0.90)
JSI METAL= 0.62 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
12/10
S. KATSOULAKOS
PROVINCE OF ONTARIO

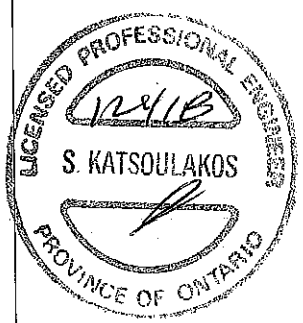
DWG NO. TAM 5115 -18
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285955	T208A	1	1	TRUSS DESC.	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
W	16-0-4	-174	-174	--	BACK	VERT	TOTAL



DWG NO. TAM 5115-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285955	T209A	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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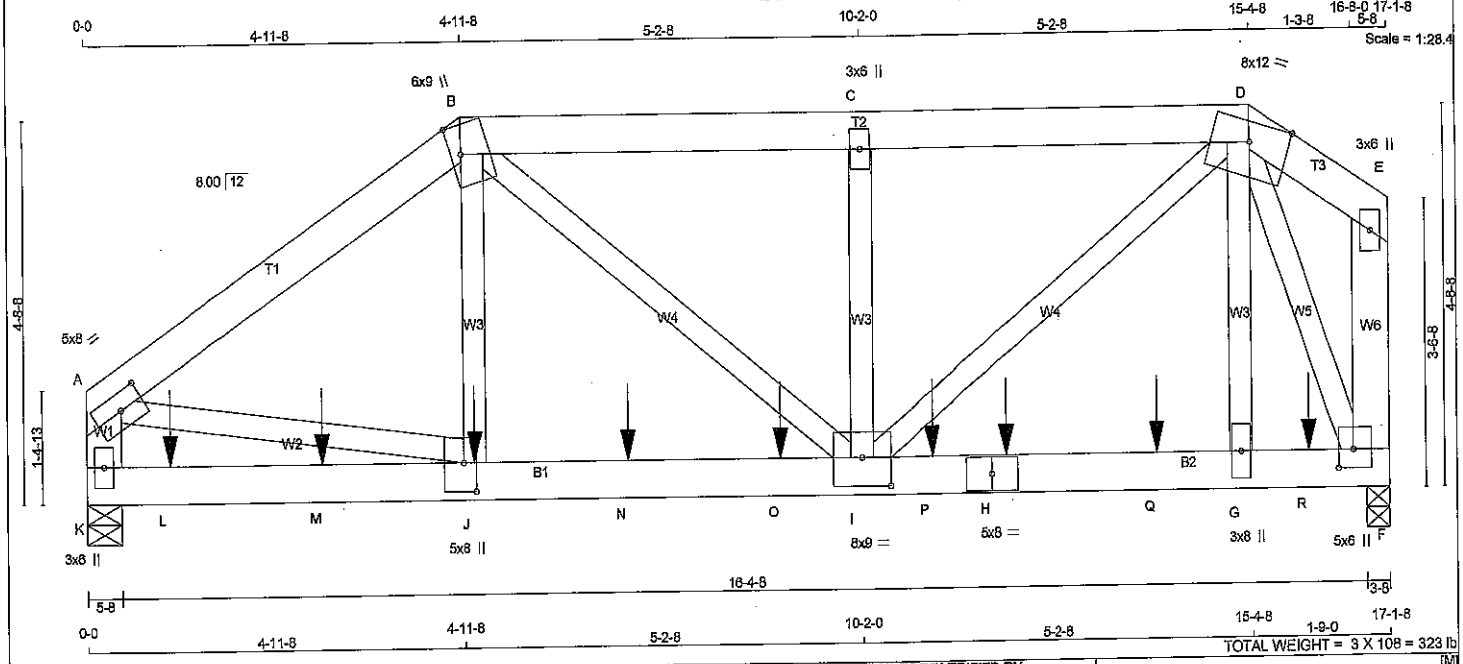


TOTAL WEIGHT = 3 X 108 = 323 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x6	DRY	No.2	SPF
B - D	2x6	DRY	No.2	SPF
D - E	2x6	DRY	No.2	SPF
K - A	2x6	DRY	No.2	SPF
F - E	2x6	DRY	No.2	SPF
K - H	2x6	DRY	2100F 1.8E	SPF
H - F	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- B	2	12	TOP
B- D	2	12	TOP
D- E	2	12	TOP
K- A	2	12	TOP
F- E	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
K- H	2	6	SIDE(692.3)
H- F	2	7	SIDE(446.6)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	8.0	2.50	3.75
B	TTWW+m	MT20	6.0	9.0	4.25	1.50
C	TMW+w	MT20	3.0	6.0		
D	TTWWW-m	MT20	8.0	12.0	Edge	
E	TMV+p	MT20	3.0	8.0		
F	BMVW1+p	MT20	5.0	6.0	2.75	2.25
G	BMW+w	MT20	3.0	8.0		
H	BS-t	MT20	5.0	8.0		
I	BMWWW-t	MT20	8.0	9.0	4.25	4.50
J	BMWW+t	MT20	5.0	8.0	4.25	2.00
K	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	8737	0	8737	0	0	5-8	5-8
F	9050	0	9050	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	7057	4075 / 0	1520 / 0	0 / 0	0 / 0	1491 / 0	0 / 0
F	7345	4217 / 0	1580 / 0	0 / 0	0 / 0	1548 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

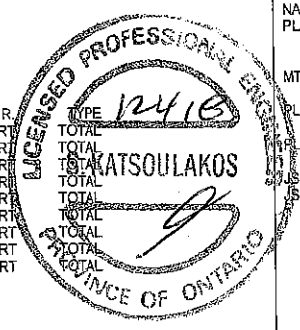
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-9664 / 0	-64.3	-84.3	0.11 (1)		4.66
B- C	-9640 / 0	-84.3	-84.3	0.11 (1)		4.66
C- D	-9640 / 0	-84.3	-84.3	0.11 (1)		4.66
D- E	0 / 0	-84.3	-84.3	0.01 (1)		10.00
K- A	-7019 / 0	0.0	0.0	0.15 (1)		6.72
F- E	-74 / 0	0.0	0.0	0.00 (1)		7.81
K- L	0 / 0	-28.0	-28.0	0.23 (1)		10.00
L- M	0 / 0	-28.0	-28.0	0.23 (1)		10.00
M- J	0 / 0	-28.0	-28.0	0.23 (1)		10.00
J- N	0 / 8159	-28.0	-28.0	0.41 (1)		10.00
N- O	0 / 8169	-28.0	-28.0	0.41 (1)		10.00
O- I	0 / 8169	-28.0	-28.0	0.41 (1)		10.00
I- P	0 / 3854	-28.0	-28.0	0.34 (1)		10.00
P- H	0 / 3854	-28.0	-28.0	0.34 (1)		10.00
H- Q	0 / 3854	-28.0	-28.0	0.34 (1)		10.00
Q- G	0 / 3854	-28.0	-28.0	0.34 (1)		10.00
G- R	0 / 3717	-28.0	-28.0	0.25 (1)		10.00
R- F	0 / 3717	-28.0	-28.0	0.25 (1)		10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- B	0 / 3738	0.20 (1)
B- I	0 / 1893	0.10 (1)
I- C	-358 / 39	0.02 (1)
I- D	0 / 7450	0.40 (1)
G- D	0 / 3950	0.21 (1)
A- J	0 / 8163	0.44 (1)
D- F	-9715 / 0	0.69 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
H	12-1-4	-1712	-1712	—	FRONT	VERT	TOTAL
J	5-1-4	-1826	-1826	—	FRONT	VERT	TOTAL
L	1-1-4	-1712	-1712	—	FRONT	VERT	TOTAL
M	3-1-4	-1712	-1712	—	FRONT	VERT	TOTAL
N	7-1-4	-1826	-1826	—	FRONT	VERT	TOTAL
O	9-1-4	-1826	-1826	—	FRONT	VERT	TOTAL
P	11-1-4	-1826	-1826	—	FRONT	VERT	TOTAL
Q	14-1-4	-1712	-1712	—	FRONT	VERT	TOTAL
R	16-1-4	-1712	-1712	—	FRONT	VERT	TOTAL



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.57")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.57")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.15/1.00 (A-K:1) , BC=0.41/1.00 (I-J:1) , WB=0.69/1.00 (D-F:1) , SSI=0.64/1.00 (G-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.88 (J) (INPUT = 0.90)
SI METAL= 0.71 (J) (INPUT = 1.00)

P6 1/2

DWG NO. TAM5116 -T8
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285955	T209A	1	3	TRUSS DESC.	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1711.9 lbs FACTORED DOWN AT 1-1-4, 1711.9 lbs FACTORED DOWN AT 3-1-4, 1826.2 lbs FACTORED DOWN AT 5-1-4, 1826.2 lbs FACTORED DOWN AT 7-1-4, 1826.2 lbs FACTORED DOWN AT 9-1-4, 1826.2 lbs FACTORED DOWN AT 11-1-4, 1711.9 lbs FACTORED DOWN AT 12-1-4, AND 1711.9 lbs FACTORED DOWN AT 14-1-4, AND 1711.9 lbs FACTORED DOWN AT 16-1-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 5116 -12
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285952	T210	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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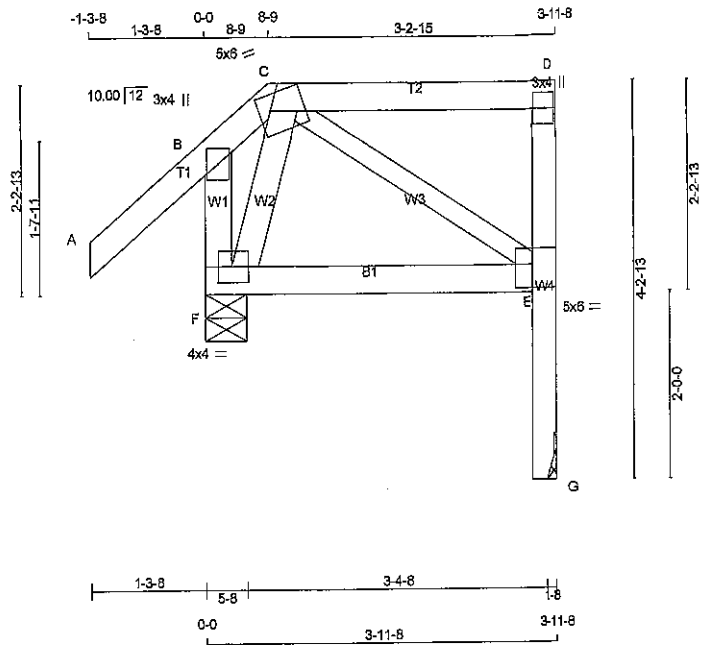


Scale = 1:24.6

TOTAL WEIGHT = 2 X 22 = 43 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
G - D	2x4	DRY	No.2	SPF
F - B	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
C - E	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TTWW-m	MT20	5.0	6.0	2.00	1.50
D	TMV+p	MT20	3.0	4.0		
E	BVMW-l	MT20	5.0	6.0	3.00	2.50
F	BMVW1-t	MT20	4.0	4.0	2.00	1.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	202	0	202	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
F	359	0	359	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	169	89 / -12	42 / 0	0 / 0	0 / 0	36 / 0	0 / 0
F	275	185 / 0	42 / 0	0 / 0	0 / 0	49 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

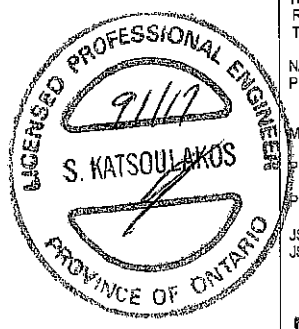
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 37	-84.3	-84.3	0.12 (1)	10.00		F- C	-54 / 81	0.01 (5)
B- C	-66 / 0	-84.3	-84.3	0.11 (1)	6.25		C- E	-24 / 35	0.01 (5)
C- D	0 / 0	-84.3	-84.3	0.15 (1)	10.00				
G- E	-202 / 0	0.0	0.0	0.03 (1)	7.81				
E- D	-134 / 0	0.0	0.0	0.02 (1)	7.81				
F- B	-253 / 0	0.0	0.0	0.03 (1)	7.81				
F- E	-30 / 20	-28.0	-28.0	0.13 (3)	6.25				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.15 (C-D:1) , BC=0.13 (E-F:3) , WB=0.01 (C-F:5) , SSI=0.10 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (F) (INPUT = 0.90)
JSI METAL= 0.07 (C) (INPUT = 1.00)

DWG NO. TAM 45611-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285952	T211	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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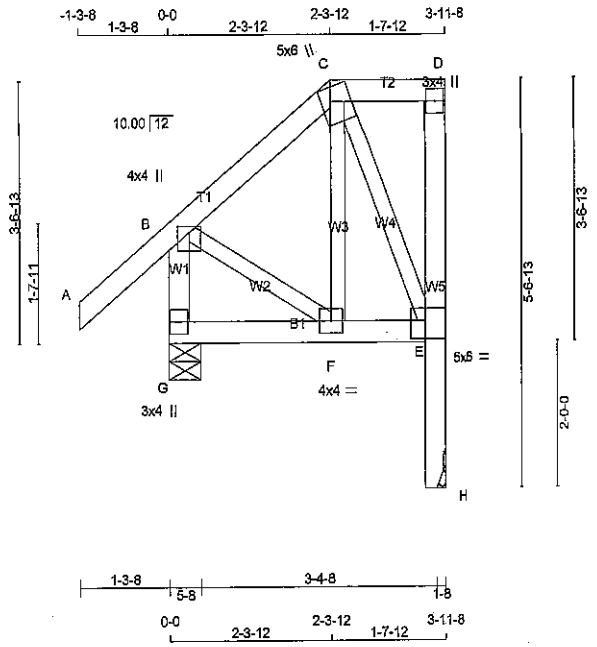

TOTAL WEIGHT = 2 X 26 = 52 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
H - D	2x4	DRY	No.2	SPF
G - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMV+p	MT20	3.0	4.0		
E	BVMW-I	MT20	5.0	6.0	3.00	2.50
F	BMWW-t	MT20	4.0	4.0		
G	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	222	0	222	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
G	339	0	339	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	182	101 / 0	42 / 0	0 / 0	0 / 0	40 / 0	0 / 0
G	262	172 / 0	42 / 0	0 / 0	0 / 0	48 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 37	-84.3	-84.3	0.12 (1)	10.00		F- C	0 / 100	0.02 (3)
B- C	-90 / 0	-84.3	-84.3	0.06 (1)	6.25		C- E	-154 / 0	0.03 (1)
C- D	0 / 0	-84.3	-84.3	0.04 (1)	10.00		B- F	0 / 78	0.02 (1)
H- E	-222 / 0	0.0	0.0	0.03 (1)	7.81				
E- D	-69 / 0	0.0	0.0	0.02 (1)	7.81				
G- B	-309 / 0	0.0	0.0	0.03 (1)	7.81				
G- F	0 / 0	-28.0	-28.0	0.04 (2)	10.00				
F- E	0 / 70	-28.0	-28.0	0.04 (2)	10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.12 (A-B:1) , BC=0.04 (E-F:2) , WB=0.03 (C-E:1) , SSI=0.07 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

DWG NO. TAM45612-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285952	P6	5	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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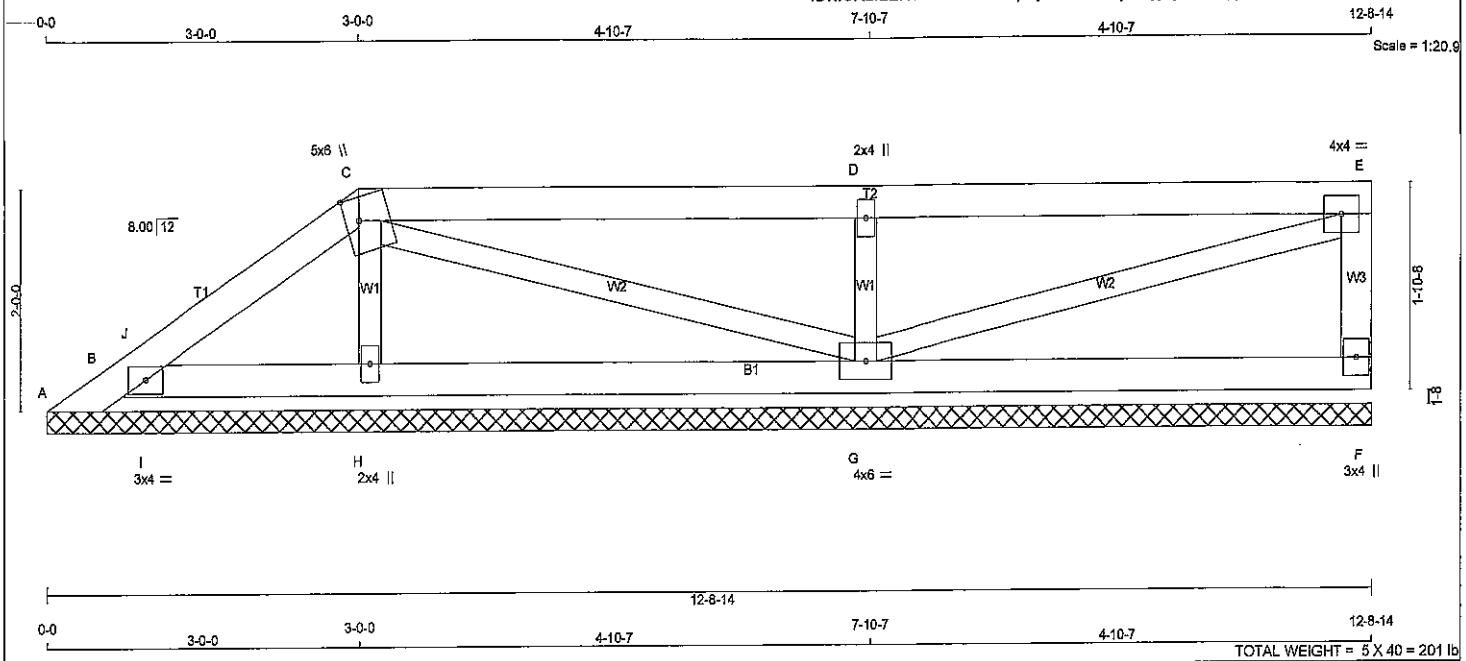

TOTAL WEIGHT = 5 X 40 = 201 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

BEARING NOTE: GAP BETWEEN INSIDE OF TOP CHORD BEARING AND FIRST DIAGONAL OR VERTICAL WEB SHALL NOT EXCEED 0.5 INCHES.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW+m	MT20	5.0	6.0	2.50	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	4.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW1-t	MT20	4.0	6.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
A	-12	0	3	0	-12	12-8-14 (11-11/16-)	
F	209	0	209	0	0	12-8-14 (11-11/16-)	
B	228	0	228	0	0	12-8-14 (11-11/16-)	
H	311	0	311	0	0	12-8-14 (11-11/16-)	
G	677	0	677	0	0	12-8-14 (11-11/16-)	

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): F , H , G

PROVIDE ANCHORAGE AT BEARING JOINT A FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	-9	0 / -9	0 / -1	0 / 0	0 / 0	2 / 0	0 / 0
F	173	94 / 0	40 / 0	0 / 0	0 / 0	38 / 0	0 / 0
B	168	122 / 0	16 / 0	0 / 0	0 / 0	30 / 0	0 / 0
H	271	125 / 0	78 / 0	0 / 0	0 / 0	67 / 0	0 / 0
G	550	315 / 0	119 / 0	0 / 0	0 / 0	116 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, F, B, H, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FR-TO	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 23		-101.8	-101.8	0.05 (1)	10.00	H- C	-196 / 0	0.03 (1)
B- J	-66 / 0		-84.3	-84.3	0.04 (1)	6.25	C- G	-23 / 0	0.01 (1)
J- C	-45 / 0		-84.3	-84.3	0.04 (1)	6.25	G- D	-512 / 0	0.07 (1)
C- D	0 / 2		-84.3	-84.3	0.34 (1)	10.00	G- E	-2 / 0	0.00 (1)
D- E	0 / 2		-84.3	-84.3	0.34 (1)	10.00	I- J	-62 / 6	0.00 (1)
F- E	-155 / 0		0.0	0.0	0.02 (1)	7.81			
B- I	0 / 33		-28.0	-28.0	0.03 (1)	10.00			
I- H	0 / 33		-28.0	-28.0	0.08 (2)	10.00			
H- G	0 / 20		-28.0	-28.0	0.17 (2)	10.00			
G- F	0 / 0		-28.0	-28.0	0.17 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.34 (C-D:1) , BC=0.17 (G-H:2) , WB=0.07 (D-G:1) , SSI=0.20 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (D) (INPUT = 0.90)
JSI METAL= 0.09 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
9/11/2
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM45615-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285952	P7	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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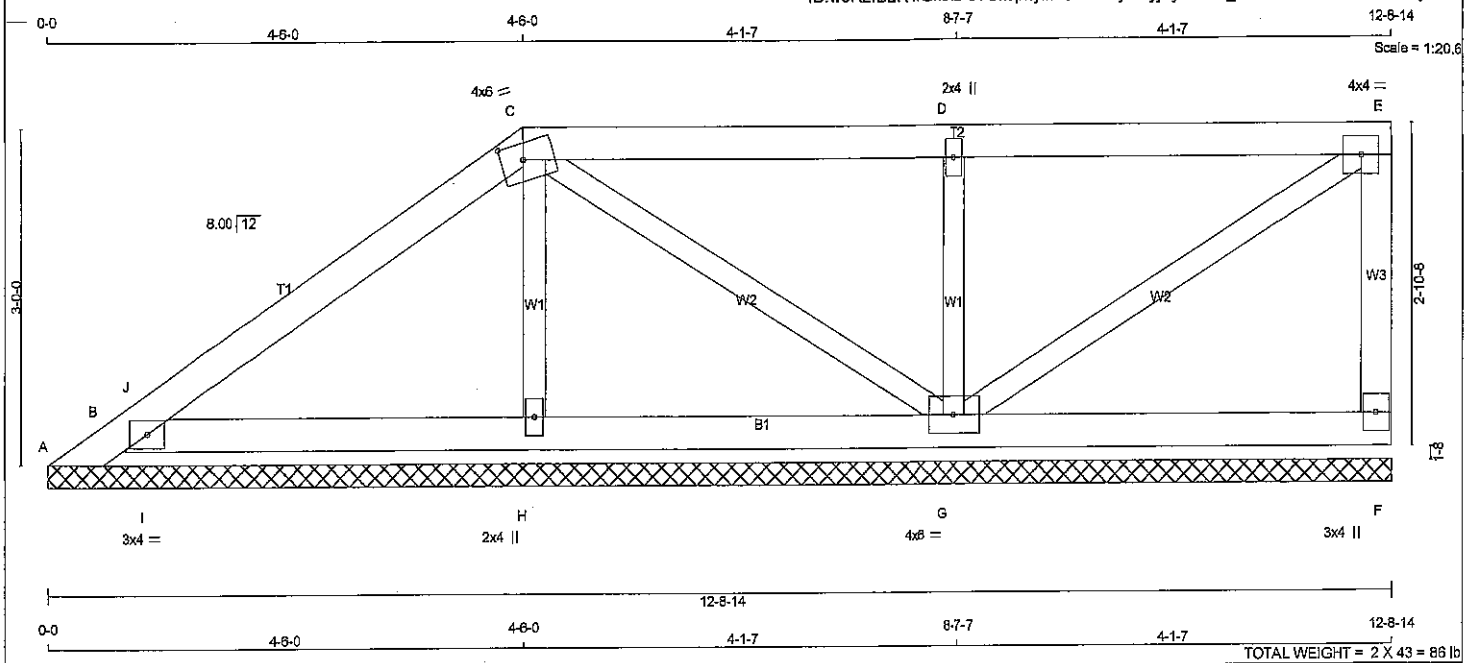


TOTAL WEIGHT = 2 X 43 = 86 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

BEARING NOTE: GAP BETWEEN INSIDE OF TOP CHORD BEARING AND FIRST DIAGONAL OR VERTICAL WEB SHALL NOT EXCEED 0.5 INCHES.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW-m	MT20	4.0	6.0	1.75	2.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	4.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW1-t	MT20	4.0	6.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
A	-142	0	0	0	-142	12-8-14 { 11-11	6-14
F	174	0	174	0	0	12-8-14 { 11-11	6-14
B	474	0	474	0	0	12-8-14 { 11-11	6-14
H	313	0	313	0	0	12-8-14 { 11-11	6-14
G	596	0	596	0	0	12-8-14 { 11-11	6-14

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): F, H, G

PROVIDE ANCHORAGE AT BEARING JOINT A FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	-116	0 / -67	0 / -27	0 / 0	0 / 0	0 / -22	0 / 0
F	145	77 / 0	35 / 0	0 / 0	0 / 0	32 / 0	0 / 0
B	373	230 / 0	87 / 0	0 / 0	0 / 0	76 / 0	0 / 0
H	273	126 / 0	79 / 0	0 / 0	0 / 0	68 / 0	0 / 0
G	479	282 / 0	98 / 0	0 / 0	0 / 0	98 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, F, B, H, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 89	-101.8	-101.8	0.17 (1)	10.00
B-J	-151 / 0	-84.3	-84.3	0.16 (1)	6.25
J-C	-69 / 0	-84.3	-84.3	0.12 (1)	6.25
C-D	0 / 9	-84.3	-84.3	0.24 (1)	10.00
D-E	0 / 9	-84.3	-84.3	0.24 (1)	10.00
F-E	-127 / 0	0.0	0.0	0.02 (1)	7.81
B-I	0 / 47	-28.0	-28.0	0.04 (3)	10.00
I-H	0 / 47	-28.0	-28.0	0.09 (2)	10.00
H-G	0 / 40	-28.0	-28.0	0.12 (2)	10.00
G-F	0 / 0	-28.0	-28.0	0.12 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H-C	-189 / 0	0.03 (1)
C-G	-58 / 0	0.02 (1)
G-D	-433 / 0	0.07 (1)
G-E	-11 / 0	0.00 (1)
I-J	-36 / 173	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.24 (C-D:1) , BC=0.12 (G-H:2) , WB=0.07 (D-G:1) , SSI=0.17 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.35 (B) (INPUT = 0.90)
JSI METAL= 0.06 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
9117
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM45616-17
STRUCTURAL
COMPONENT ONLY

LUL/LUS/LJS/HUS/HHUS/HGUS

Standard and Double-Shear Joist Hangers

This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 258–259.

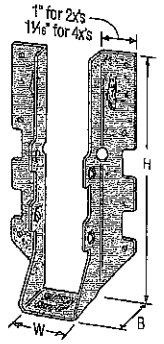
Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 20–24.

Installation:

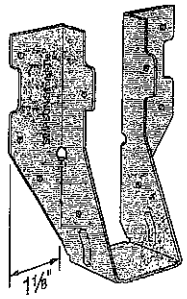
- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on p. 126.



LUS28



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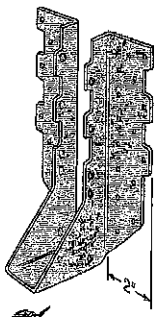
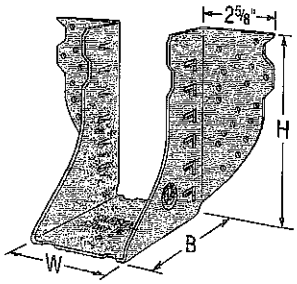
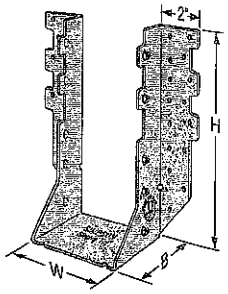


HUS210
(HUS26, HUS28, and HHUS similar)



HGUS28-2



HHUS210-2

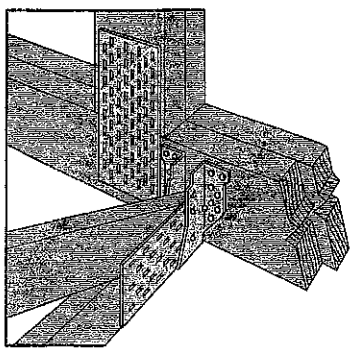
Double-Shear Nailing Top View

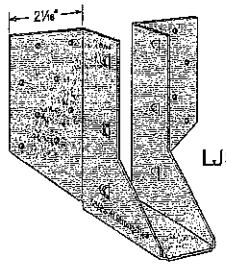
Double-Shear Nailing Side View; Do not bend tab

Dome Double-Shear Nailing Side View *(available on some models)*

U.S. Patent 5,603,580

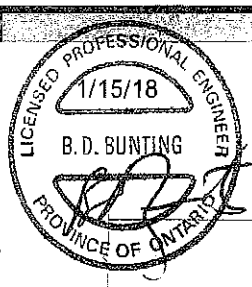
Typical HUS26 Installation with Reduced Heel Height
(Truss Designer to provide fastener quantity for connecting multiple members together)





LJS26DS

LUL/LUS/LJS/HUS/HHUS/HGUS



HHUS/HGUS

See Hanger Options information on pp. 125–127.

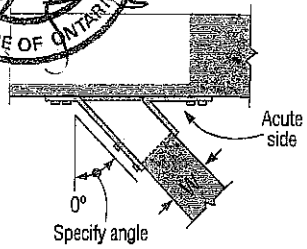
HHUS — Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.62 of the table value
- The joist must be bevel-cut to allow for double-shear nailing

HGUS — Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Joist	Down Resistance	Uplift
W < 2"	Bevel or square cut	0.62 of table value	0.46 of table value
2" < W < 6"	Bevel cut	0.67 of table value	0.41 of table value
2" < W < 6"	Square cut	0.46 of table value	0.41 of table value
W > 6"	Bevel cut	0.75 of table value	0.41 of table value



Top View HHUS Hanger Skewed Right
(joist must be bevel cut)
All joist nails installed on the outside angle *(non-acute side)*.

Standard and Double-Shear Joist Hangers (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.) W	H	B	d_e[3]	Fasteners Header	Joist	Factored Resistance D.Fir-L Uplift (K_D = 1.15) lb. / kN	D.Fir-L Normal (K_D = 1.00) lb. / kN	S-P-F Uplift (K_D = 1.15) lb. / kN	S-P-F Normal (K_D = 1.00) lb. / kN
Single 2x Sizes											
LUS24	18	1⁹⁄₁₆	3⅛	1¾	2¼	(4) 10d	(2) 10d	710 / 3.16	1625 / 7.23	645 / 2.87	1155 / 5.14
LU24L	22	1⁹⁄₁₆	3	1⅝	2¹⁵⁄₁₆	(4) 10d	(2) 10d x 1½"	360 / 1.60	1020 / 4.54	320 / 1.42	725 / 3.22
LU26L	22	1⁹⁄₁₆	5	1⅝	4⅝	(6) 10d	(4) 10d x 1½"	720 / 3.20	1605 / 7.14	645 / 2.87	1140 / 5.07
LUS26	18	1⁹⁄₁₆	4¾	1¾	3¾	(4) 10d	(4) 10d	1420 / 6.32	2170 / 9.65	1290 / 5.74	1630 / 7.25
HUS26	16	1⅝	5⅞	3	3¹⁵⁄₁₆	(14) 16d	(6) 16d	2705 / 11.30	4940 / 21.97	2065 / 9.20	3875 / 17.24
LJS26DS	18	1⁹⁄₁₆	5	3½	4⅝	(18) 16d	(6) 16d	2055 / 9.14	4265 / 18.97	1460 / 6.49	4115 / 18.31
HGUS26	12	1⅝	5⅞	5	4⅛	(20) 16d	(8) 16d	2685 / 11.96	6625 / 29.51	2685 / 11.96	5700 / 25.35
LU28L	20	1⁹⁄₁₆	6¾	1⅝	5⅞	(8) 10d	(6) 10d x 1½"	1140 / 5.07	2185 / 9.72	1020 / 4.54	1550 / 6.89
LUS28	18	1⁹⁄₁₆	6⅝	1¾	3¾	(6) 10d	(4) 10d	1420 / 6.32	2520 / 11.21	1290 / 5.74	1790 / 7.96
HUS28	16	1⅝	7⅛	3	6⅛	(22) 16d	(8) 16d	3605 / 16.04	5365 / 23.86	2675 / 11.90	4345 / 19.33
HGUS28	12	1⅝	7⅛	5	6⅛	(36) 16d	(12) 16d	3310 / 14.74	7675 / 34.19	3310 / 14.74	6900 / 30.73
LU210L	20	1⁹⁄₁₆	8	1⅝	7⅝	(10) 10d	(6) 10d x 1½"	1140 / 5.07	2495 / 11.10	1020 / 4.54	1770 / 7.87
LUS210	18	1⁹⁄₁₆	7¹³⁄₁₆	1¾	3⅞	(8) 10d	(4) 10d	1420 / 6.32	2785 / 12.39	1290 / 5.74	2210 / 9.83

1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.
3. d_e is the distance from the bearing seat to the top joist nail.
4. Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-C-N10TRSSCN and/or see installation notes.
5. **Nails:** 16d = 0.162" dia. x 3½" long. See pp. 27–28 for other nail sizes and information.

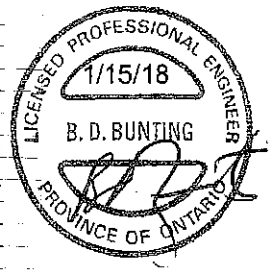
Face-Mount Hangers

SIMPSON Strong-Tie®

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

Model No.	Ga.	Dimensions (In.) W	H	B	d_s[3]	Fasteners Header	Joist	Factored Resistance D.Fir-L Uplift (K_D = 1.15) lb. / kN	D.Fir-L Normal (K_D = 1.00) lb. / kN	S-P-F Uplift (K_D = 1.15) lb. / kN	S-P-F Normal (K_D = 1.00) lb. / kN
Double 2x Sizes											
LUS24-2	18	3⅛	3⅛	2	1½	(4) 16d	(2) 16d	835 3.71	2020 8.99	590 2.62	1435 6.38
LUS26-2	18	3⅛	4⅞	2	4	(4) 16d	(4) 16d	1720 7.65	2595 11.54	1545 6.87	1920 8.54
HHUS26-2	14	3⁹⁄₁₆	5⅜	3	3¹⁵⁄₁₆	(14) 16d	(6) 16d	2850 12.68	7335 32.63	2065 9.20	5205 23.15
HGUS26-2	12	3⁹⁄₁₆	5⅞	4	4⅛	(20) 16d	(8) 16d	4385 19.51	8950 39.81	3110 13.83	6355 28.27
LUS28-2	18	3⅛	7	2	4	(6) 16d	(4) 16d	1720 7.65	3325 14.79	1545 6.87	2575 11.45
HHUS28-2	14	3⁹⁄₁₆	7⅝	3	6⅛	(22) 16d	(8) 16d	3765 16.75	8940 39.77	2675 11.90	6345 28.22
HGUS28-2	12	3⁹⁄₁₆	7⅞	4	6⅛	(36) 16d	(12) 16d	6070 27.00	12980 57.74	4310 19.17	9215 40.99
LUS210-2	18	3⅛	9	2	6	(8) 16d	(6) 16d	2580 11.48	4500 20.02	2320 10.32	3195 14.21
HHUS210-2	14	3⁹⁄₁₆	9⅜	3	8	(30) 16d	(10) 16d	4670 20.77	9660 42.97	4235 18.84	7000 31.14
HGUS210-2	12	3⁹⁄₁₆	9⅜	4	8⅛	(46) 16d	(16) 16d	6840 30.43	14015 62.34	4855 21.60	10270 45.69
Triple 2x Sizes											
HGUS26-3	12	4¹⁵⁄₁₆	5½	4	4⅛	(20) 16d	(8) 16d	4385 19.51	8950 39.81	3110 13.83	6355 28.27
HGUS28-3	12	4¹⁵⁄₁₆	7¼	4	6⅛	(36) 16d	(12) 16d	6070 27.00	12980 57.74	4310 19.17	9215 40.99
HHUS210-3	14	4¹⁄₁₆	9	3	7¹⁵⁄₁₆	(30) 16d	(10) 16d	4670 20.77	9670 43.02	4235 18.84	6865 30.54
HGUS210-3	12	4¹⁵⁄₁₆	9¼	4	8⅛	(46) 16d	(16) 16d	6840 30.43	14645 65.14	4855 21.60	10400 46.26
Quadruple 2x Sizes											
HGUS26-4	12	6⁹⁄₁₆	5⅞	4	4⅛	(20) 16d	(8) 16d	4385 19.51	8950 39.81	3110 13.83	6355 28.27
HGUS28-4	12	6⁹⁄₁₆	7⅝	4	6⅛	(36) 16d	(12) 16d	6070 27.00	12980 57.74	4310 19.17	9215 40.99
HHUS210-4	14	6⅛	8⅞	3	7¹³⁄₁₆	(30) 16d	(10) 16d	4670 20.77	10155 45.17	4235 18.84	7210 32.07
HGUS210-4	12	6⁹⁄₁₆	9⅜	4	8⅛	(46) 16d	(16) 16d	6840 30.43	14645 65.14	4855 21.60	10400 46.26
HGUS212-4	12	6⁹⁄₁₆	10⅝	4	10⅛	(56) 16d	(20) 16d	7640 33.98	14995 66.70	5425 24.13	10645 47.35
HGUS214-4	12	6⁹⁄₁₆	12⅝	4	11⅛	(66) 16d	(22) 16d	10130 45.06	16400 72.95	7195 32.00	11645 51.80
4x Sizes											
LUS46	18	3⁹⁄₁₆	4¾	2	3⅞	(4) 16d	(4) 16d	1720 7.65	2595 11.54	1545 6.87	1920 8.54
HHUS46	14	3⅝	5¼	3	3¹⁵⁄₁₆	(14) 16d	(6) 16d	2540 11.30	7335 32.63	2065 9.20	5205 23.15
HGUS46	12	3⅝	5¼	4	4⅛	(20) 16d	(8) 16d	4385 19.51	8950 39.81	3110 13.83	6355 28.27
LUS48	18	3⁹⁄₁₆	6¾	2	3⅞	(6) 16d	(4) 16d	1720 7.65	3325 14.79	1545 6.87	2575 11.45
HHUS48	14	3⅝	7⅛	3	6⅛	(22) 16d	(8) 16d	3765 16.75	8940 39.77	2675 11.90	6345 28.22
HGUS48	12	3⅝	7⅛	4	6⅛	(36) 16d	(12) 16d	6070 27.00	12980 57.74	4310 19.17	9215 40.99
LUS410	18	3⁹⁄₁₆	8¾	2	5⁹⁄₁₆	(8) 16d	(6) 16d	2580 11.48	4500 20.02	2320 10.32	3195 14.21
HGUS410	12	3⅝	9	4	8⅛	(46) 16d	(16) 16d	6840 30.43	14015 62.34	4855 21.60	10270 45.69
HGUS412	12	3⅝	10⅞	4	10⅛	(56) 16d	(20) 16d	7640 33.98	14995 66.70	5425 24.13	10645 47.35
HGUS414	12	3⅝	12⅞	4	11⅛	(66) 16d	(22) 16d	10130 45.06	16400 72.95	7195 32.00	11645 51.80



See footnotes on p. 258.

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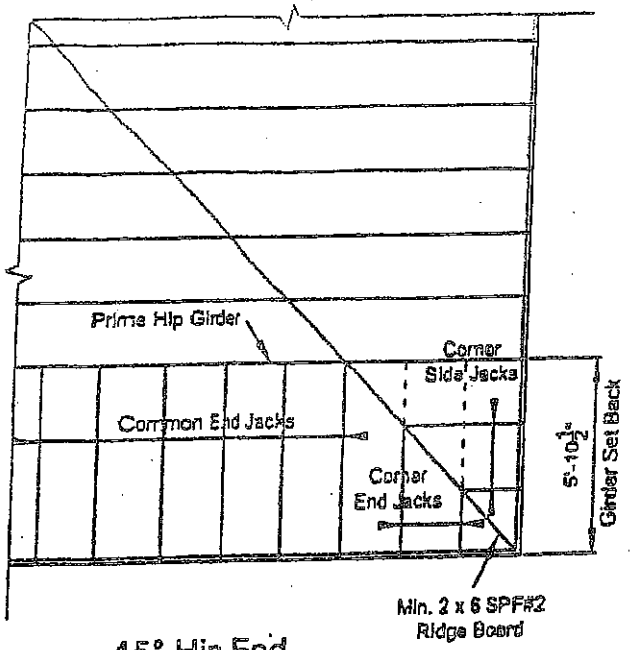


45° Hip End

LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2

BOTTOM CHORD : 2 x 4 SPF#2

WEBS : 2 x 3 SPF#2

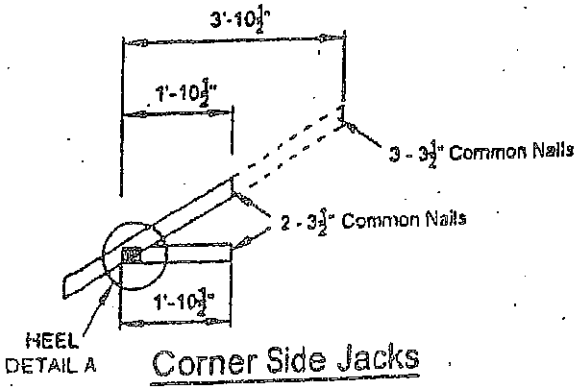
UNLESS OTHERWISE SHOWN

DESIGN LOAD:

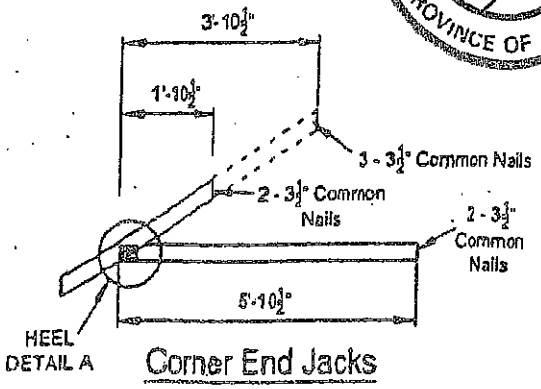
TOP CHORD LIVE LOAD	: 34.8 P.S.F.
TOP CHORD DEAD LOAD	: 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	: 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	: 7.0 P.S.F.
TOTAL LOAD	: 44.8 P.S.F.

DWG NO TAM 3495.14 R4
STRUCTURAL COMPONENT ONLY

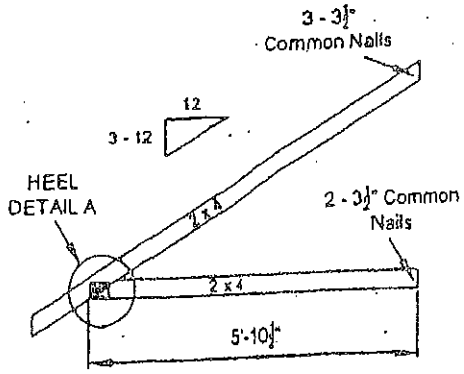
LICENSED PROFESSIONAL ENGINEER
1-2016
S. KATSOULAKOS
PROVINCE OF ONTARIO



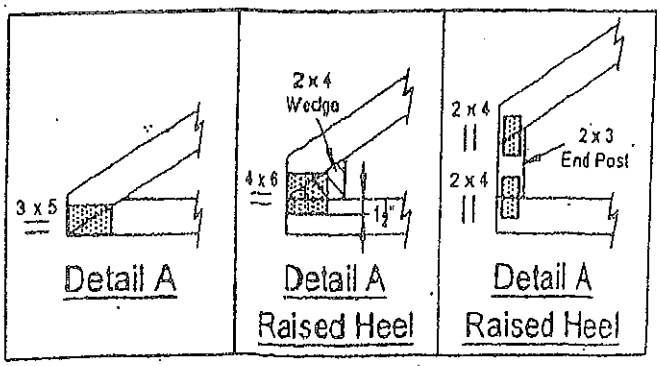
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

MICRO CITY
ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

DESIGN LOAD:

TOP CHORD LIVE LOAD	: 40.5 P.S.F.
TOP CHORD DEAD LOAD	: 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	: 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	: 7.0 P.S.F.
TOTAL LOAD	: 50.5 P.S.F.

DWG NO TAM3504.1
STRUCTURAL
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1-2014
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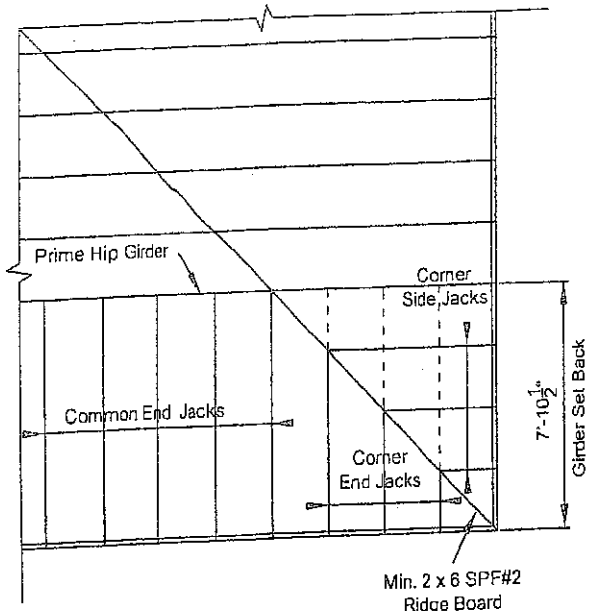
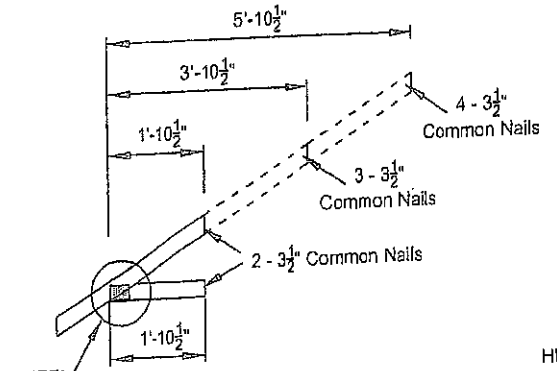
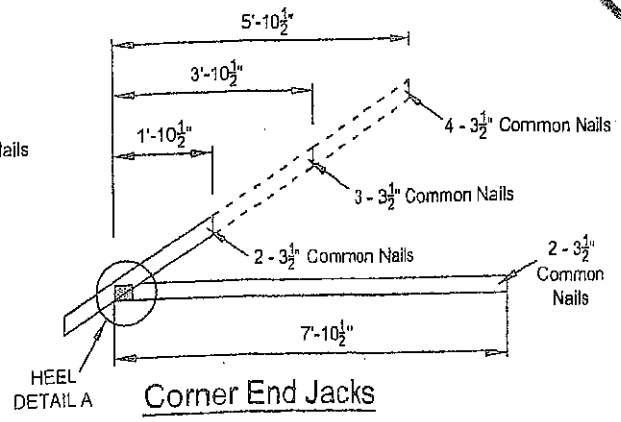
PROVINCE OF ONTARIO



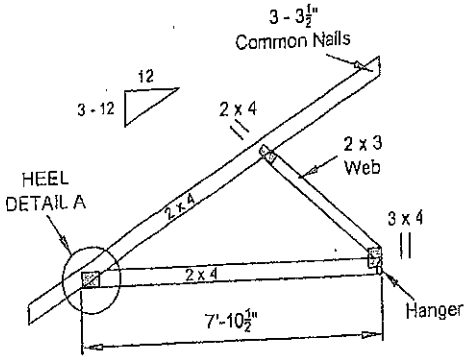
45° Hip End



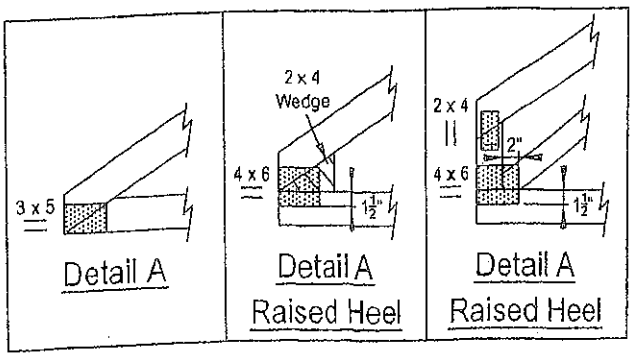
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
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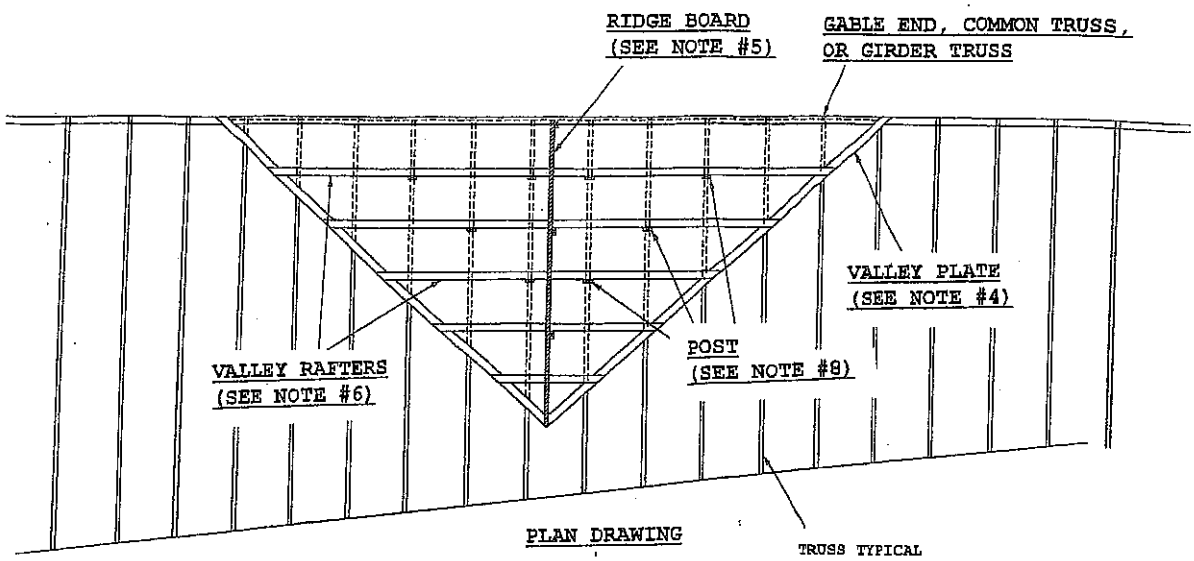
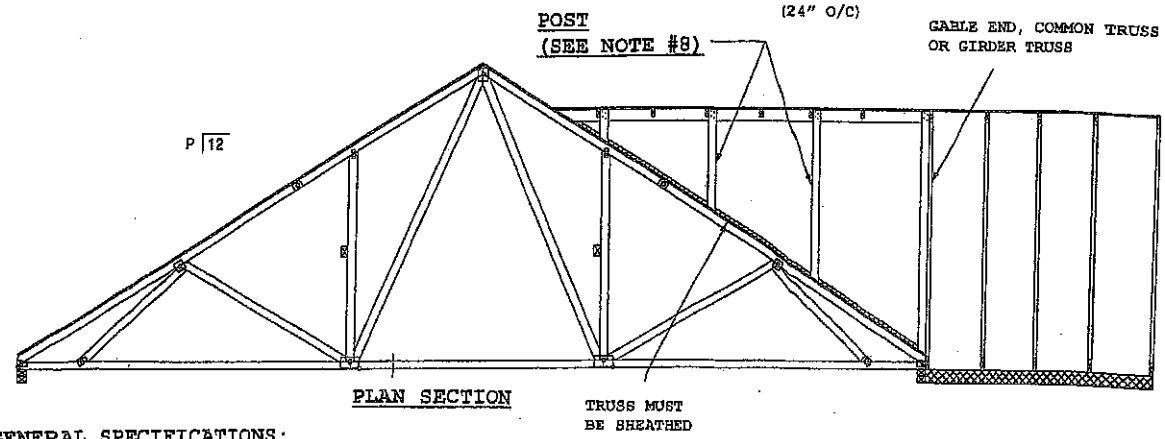
CONVENTIONAL VALLEY FRAMING DETAIL



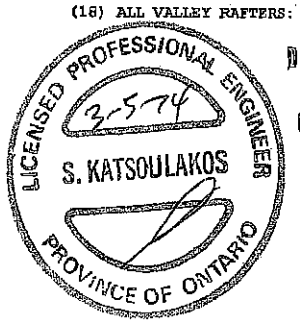


GENERAL SPECIFICATIONS:

(1) WITH THE BASE TRUSSES ERECTED (INSTALLED), APPLY SHEATHING TOP CHORD OF SUPPORTING (BASE) TRUSSES.
(2) BRACE BOTTOM CHORD AND WEB MEMBERS AS PER PRE-ENGINEERED TRUSS DESIGNS.
(3) DEFINE VALLEY RIDGE BY RUNNING A LEVEL STRING FROM THE INTERSECTING RIDGE OF THE (a) GABLE END, (b) GIRDER TRUSS OR (c) COMMON TRUSS TO THE ROOF SHEATHING.
(4) INSTALL 2 X 6 VALLEY PLATES ON FLAT. FASTEN TO EACH SUPPORTING TRUSS WITH (2) 16d (3.5" X 0.131") NAILS.
(5) SET A 2 X 6 #2 RIDGE BOARD (MAX. 10'-0" RIDGE) OR 2 X 8 #2 SPF RIDGE BOARD (MAX. 20'-0" RIDGE). SUPPORT RIDGE BOARD WITH 2 X 4 POSTS SPACED 48" O/C. BEVEL BOTTOM OF POST TO SET EVENLY ON THE SHEATHING. FASTEN POST TO RIDGE WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST TO ROOF SHEATHING WITH (3) 10d (3" X 0.131") TOE-NAILS.
(6) FRAME VALLEY RAFTERS FROM VALLEY PLATE TO RIDGE BOARD. MAXIMUM RAFTER SPACING IS 24" O/C. FASTEN VALLEY RAFTER TO RIDGE BEAM WITH (3) 16d (3.5" X 0.131") TOE-NAILS. FASTEN VALLEY RAFTER TO VALLEY PLATE WITH (3) 16d (3.5" X 0.131") TOE-NAILS.
(7) SUPPORT THE VALLEY RAFTERS WITH 2 X 4 POSTS AT 48" O/C (OR LESS) ALONG EACH RAFTER. INSTALL POSTS IN A STAGGERED PATTERN AS SHOWN ON PLAN DRAWING. ALIGN POSTS WITH TRUSSES BELOW. FASTEN VALLEY RAFTER TO POST WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST THROUGH SHEATHING TO SUPPORTING TRUSSES WITH (2) 16d (3.5" X 0.131") NAILS.
(8) POSTS SHALL BE 2 X 4 #2 SPF OR BETTER. POSTS EXCEEDING 75" IN HEIGHT SHALL BE INCREASED TO 4 X 4 #2 SPF, OR BETTER, OR BE PRE-ASSEMBLED TWO (2) PLY 2 X 4 #2 SPF OR BETTER FASTENED TOGETHER WITH 2 ROWS OF 10d (3" X 0.131") NAILS AT 6" O/C.
(9) MAINTAIN A MINIMUM 3/4" LUMBER EDGE DISTANCE WHEN NAILING. NAIL SPACING SHOULD APPROXIMATE A MINIMUM 1-3/4" O/C OR MORE UNLESS NOTED OTHERWISE. ALL CONSTRUCTION TO CONFORM TO ONTARIO BUILDING CODE (CURRENT ADDITION) AT ALL TIMES.

NOTES:

(10) 48" O/C (MAXIMUM POST SPACING.
(11) ROOF LIVE LOAD = 34.8 PSF (MAX.)
(12) ROOF DEAD LOAD = 10.0 PSF (MAX.)
(13) PART 9 APPLICATION ONLY (ONTARIO BUILDING CODE)
(14) PART 4 APPLICATION ONLY (ONTARIO BUILDING CODE) WITH APPROVED REVIEW BY LICENSED PROFESSIONAL ENGINEER.
(15) BASE TRUSS SPACING (24" O/C MAX.)
(16) ALL PRE-ENGINEERED BASE TRUSS COMPONENTS TO BE SEALED BY LICENSED PROFESSIONAL ENGINEER AND THIS DETAIL TO BE VERIFIED AND APPROVED BY SAME WHEN RIDGE BOARD LENGTH EXCEEDS 12'-0".
(17) ALL BASE TRUSSES: P = 4 (4/12) - MINIMUM.
(18) ALL VALLEY RAFTERS: P = 4 (4/12) - MINIMUM.



DWG NO TAM6305.14
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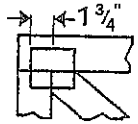
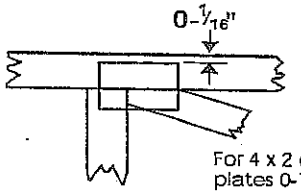
Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

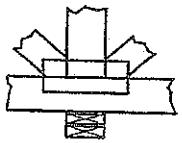
4 X 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

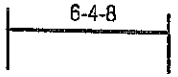
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses

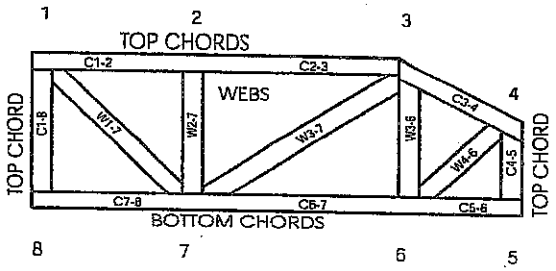
DSB-89: Design Standard for Bracing.

BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

· CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

MiTek®

POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MII-7473C rev. 10-'08

⚠ General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

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Glencoe, Ontario
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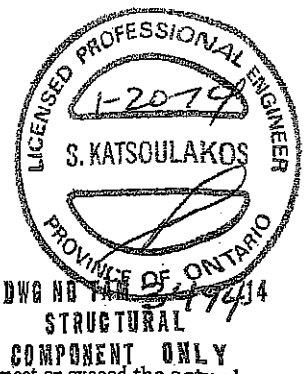
(519) 287 - 2242; Fax: (519) 287 - 5750 (Call)



Responsibilities:

Micro City Engineering Services is responsible for the design of trusses as individual components.

It is the responsibilities of others to ascertain that the design loads utilized on this (these) drawing(s) meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdiction over such decisions.

All dimensions are to be verified by the owner, contractor, architect, or other authority having input over such decisions prior to truss component manufacture. At no time shall Micro City Engineering Services Inc. or its employees be responsible for dimension errors.

Micro City Engineering Services Inc. bears no responsibility for the erection of any truss components. Persons erecting truss components are cautioned to seek professional advice regarding temporary and permanent bracing systems and to be **totally** familiar with all aspects of truss erection prior to proceeding on **any** truss component erection job. Any bracing shown on Micro City Engineering Services Inc. or Tamarack Roof Trusses Inc. sealed or unsealed truss component drawings is specified for the single truss component in question and is identified as an integral part of the design for that particular truss component but is **not** meant to represent the only required bracing for that particular truss component when installed as a component in a series of truss components in a roof truss system.

It is the truss manufacturer's responsibility to ensure that trusses are manufactured in accordance with Micro City Engineering Services Inc. specifications outlined below:

SPECIFICATIONS:

Truss components sealed by Micro City Engineering Services Inc. must conform to the relevant sections of the current Building Code of Ontario and Canada (Part 4 or Part 9) or the current Farm Building Code of Canada in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the Truss Plate Institute of Canada (TPIC). All unit lumber and nailing stresses identified on truss component design drawings and/or used in the design of individual truss components shall conform to the current CSA Wood Design standard identified in the current Building Code and TPIC Design Standards.

The lumber used to manufacture any truss component is to conform to the specified size and grade identified on the truss drawing.

The lumber used in the manufacture of any truss component is not to exceed 19% during its service use unless specifically noted on the truss drawing.

The lumber used in the manufacture of any truss component is not to be treated with any chemicals during its service life unless specifically noted on the truss drawing.

Connector plates shall be applied to both faces of the truss component at each joint and shall be positioned exactly as specified.

The top chord of any truss component is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the sealed truss component drawing but not exceeding 24" o/c (Part 9 design) and not exceeding 48" o/c (Part 4 or Agricultural design).

When a truss component is to be installed with no rigid ceiling attached directly to the bottom chord, then the bottom chord is to be laterally braced at intervals not exceeding 3m (or 10'-0").

All sealed or unsealed truss component drawings provided by Micro City Engineering Services Inc. Or Tamarack Roof Trusses Inc. should be read in conjunction with the following:

Warning-Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473C rev 10-'08 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer - not the truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component Safety Information available from the Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA, 22314.