

ASPHALT SHINGLES
12" FINISH O.H.
R.T.M.C
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HIGHGROVE-3E
EL:3 - UNIT1
205862

HIGHGROVE-4
EL:3 UNIT2
285485

HIGHGROVE-2
EL:3 (REV) UNIT3
285492

HIGHGROVE-2
EL:3 UNIT4
285494

HIGHGROVE-3
EL:3 (REV) UNIT5
285864

HIGHGROVE-5E
EL:3 (REV) - UNIT6
285865

HARDWARE:

HGUS26-2 -(XX)
LJS26DS -(V)
LUS24 -(O)

8/12 PITCHES (TYP.)
UNLESS NOTED

DENOTES:

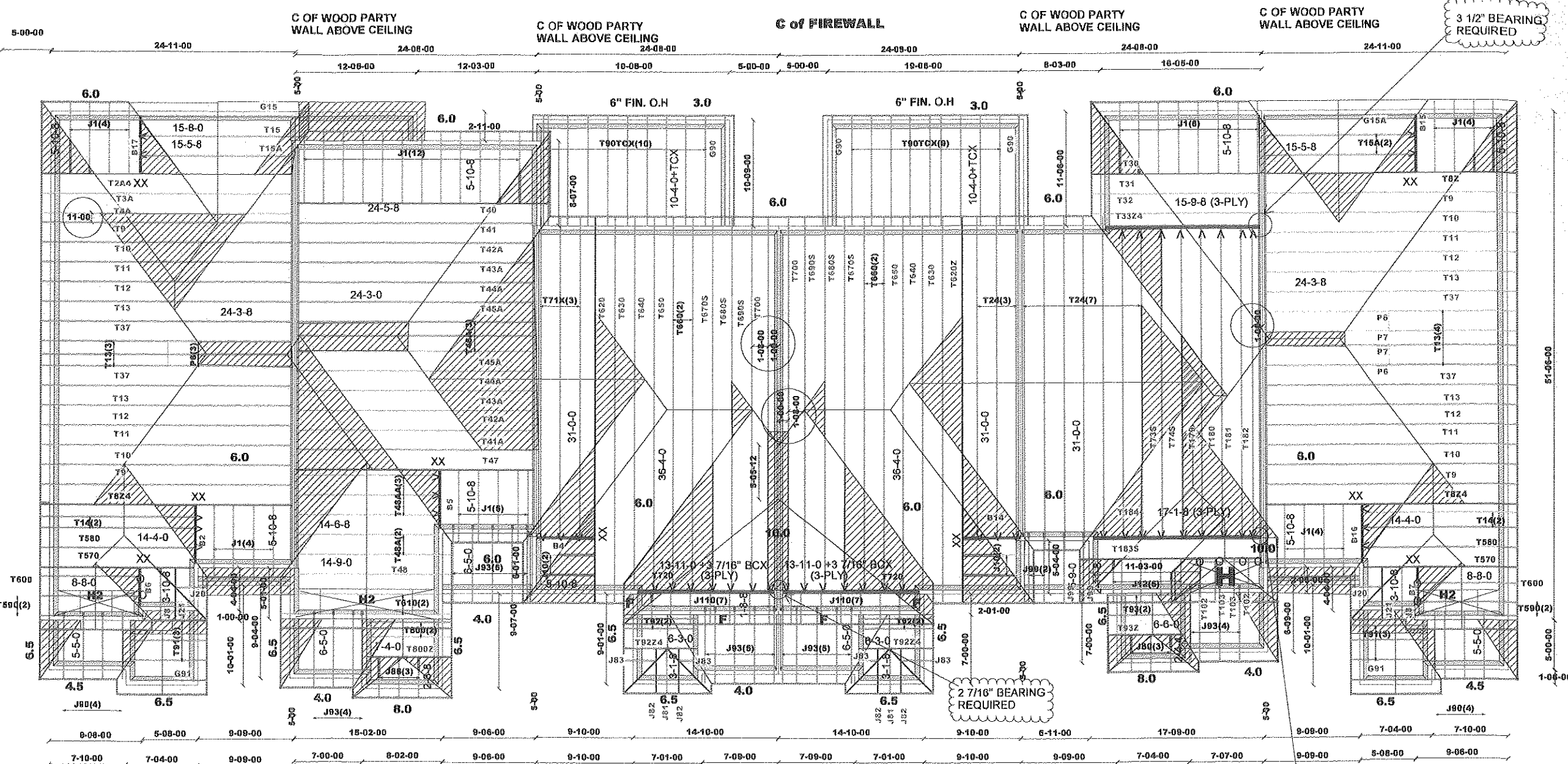
H -2'-0"-0 HIGHER PLATE
H2 -1'-6"-0 RAISED CEILING
F -12" HIGHER FACIA ONLY
ALL B- 2-2X10 BEAMS (FLUSH)

T-170534

DENOTES:
CONVENTIONAL
FRAMING

51-06-00

9-00-00
1-06-00



ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION
ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"x4"SPF@24"o.c.
WITH A 2"x4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.
POSTS LONGER THAN 6' TO BE Laterally BRACED SO THAT THE DISTANCE
BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN LOADS:

SNOW LOAD 25.6 PSF
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF



Job Track: 43954
Layout ID: 285861
Plan Log: 93446

Builder / Location:

GREENPARK HOMES / WATERDOWN

Project:

RUSSEL GARDENS PHASE 2

Date: 07/03/18

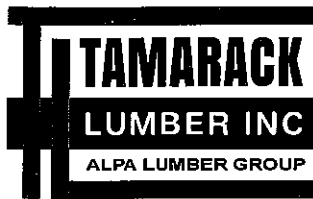
Designer: sonny

Model / Elevation:

BLOCK208/ UNIT1 TO6 (REVISED)

Mitek ver 7.5.0

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TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.



Delivery Shiplist

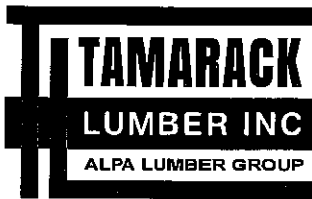
DATE	04/18/18
SALES REP	Mario

JOB TRACK: 43954	LAYOUT ID: 285862	LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH 2	SUB-BUILDER:	
MODEL: HIGHGROVE 5E	ELEVATION: 3- UNIT1	

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY PLY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
						TOP	BOT					
	1	T2A4 HIP GIRDER	8.00 0.00	24-03-08	04-01-04	2 X 6	2 X 6	01-03-08 00-00-00	01-04-13 01-06-08	142.19 87.67		
	1	T3A HIP	8.00 0.00	24-03-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	94.81 60.17		
	1	T4A HIP	8.00 0.00	24-03-08	06-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	103.53 66.33		
	1	T8Z4 HALF HIP	8.00 0.00	24-03-08	04-01-04	2 X 6	2 X 6	01-03-08 00-00-00	01-04-13 04-01-04	143.26 88.34		
	2	T9 HALF HIP	8.00 0.00	24-03-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-01-04	197.10 123.66		
	2	T10 HALF HIP	8.00 0.00	24-03-08	06-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 06-01-04	212.66 133.34		
	2	T11 HALF HIP	8.00 0.00	24-03-08	07-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 07-01-04	225.56 142.68		
	2	T12 HALF HIP	8.00 0.00	24-03-08	08-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 08-01-04	234.64 146.34		
	5	T13 HALF HIP	8.00 0.00	24-03-08	09-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 09-01-04	614.40 379.15		
	2	T37 HALF HIP	8.00 0.00	24-03-08	10-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 10-01-04	257.54 159.34		
	1	T15 COMMON	8.00 0.00	15-08-00	06-07-08	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	66.76 42.00		
	1	G15 COMMON	8.00 0.00	15-08-00	06-07-08	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	69.72 45.00		
	1	T15A COMMON	8.00 0.00	15-05-08	06-07-08	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	64.50 41.33		
	2	T14 COMMON	8.00 0.00	14-04-00	06-02-02	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	123.52 78.00		
	1	T570 HIP GIRDER	8.00 0.00	14-04-00	03-11-13	2 X 4	2 X 6	01-03-08 01-03-08	01-04-13 01-04-13	70.63 46.34		
	1	T580 HIP	8.00 0.00	14-04-00	05-03-13	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	62.73 39.33		
	2	T590 ROOF	8.00 8.00	08-08-00	04-03-08	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	83.72 58.68		
	1	T600 COMMON	8.00 0.00	08-08-00	04-03-08	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	37.48 23.67		



Delivery Shiplist

DATE	04/18/18
SALES REP	Mario

JOB TRACK:43954	LAYOUT ID: 285862	LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH 2	SUB-BUILDER:	
MODEL: HIGHGROVE 5E	ELEVATION: 3- UNIT1	

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	3	T91 COMMON	6.50 0.00	06-06-00	02-04-09	2 X 4	2 X 4	01-03-08 01-03-08	00-07-07 00-07-07	66.60 45.00		
	1	G91 COMMON	6.50 0.00	06-06-00	02-04-09	2 X 4	2 X 4	01-03-08 01-03-08	00-07-07 00-07-07	23.39 16.00		
	3	P6 PIGGYBACK	8.00 0.00	11-11-15	01-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 01-10-08	120.33 77.01		
	8	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	134.32 85.36		
	1	J8 JACK-OPEN	8.00 0.00	03-10-08	03-11-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 03-11-13	12.81 8.00		
	1	J20 JACK-OPEN	8.00 0.00	01-10-08	02-07-02	2 X 4	2 X 4	01-03-08 -00-01-01	01-04-13 00-03-08	9.17 5.67		
	1	J21 JACK-OPEN	8.00 0.00	03-10-08	02-07-02	2 X 4	2 X 4	01-03-08 -02-01-01	01-04-13 00-03-08	12.97 8.00		
	4	J90 JACK	4.50 0.00	05-05-00	02-05-00	2 X 4	2 X 4	01-03-08 00-00-00	00-04-10 02-05-00	57.88 37.32		

TOTAL # TRUSS= 51.00

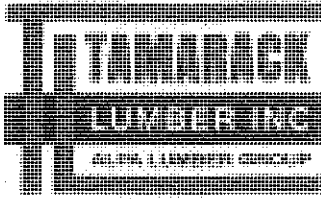
TOTAL BFT OF ALL TRUSSES=

2043.73 BFT. TOTAL WEIGHT OF ALL TRUSSES= 3242.22 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	HGUS26-2	
2	Hangers	LUS24	
5	Hangers	LJS26DS	

TOTAL # ITEMS= 10.00



Delivery Shiplist

DATE	07/03/18
SALES REP	Mario

JOB TRACK: 43954	LAYOUT ID: 285485	LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH	SUB-BUILDER:	
MODEL: HIGHGROVE 4	ELEVATION: 3- UNIT2	

ROOF TRUSSES

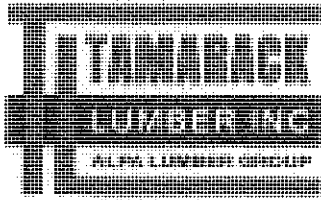
ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	T40 HALF HIP	8.00 0.00	24-05-08	04-01-04	2 X 6	2 X 6	01-03-08 00-00-00	01-04-13 04-01-04	136.90 84.33		
	1	T41 HALF HIP	8.00 0.00	24-05-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-01-04	99.05 61.83		
	1	T41A HALF HIP	8.00 0.00	24-03-00	05-01-04	2 X 4	2 X 4	00-00-00 00-00-00	01-06-08 05-01-04	96.77 61.17		
	2	T42A HALF HIP	8.00 0.00	24-03-00	06-01-04	2 X 4	2 X 4	00-00-00 00-00-00	01-06-08 06-01-04	209.24 132.00		
	2	T43A HALF HIP	8.00 0.00	24-03-00	07-01-04	2 X 4	2 X 4	00-00-00 00-00-00	01-06-08 07-01-04	222.10 141.34		
	2	T44A HALF HIP	8.00 0.00	24-03-00	08-01-04	2 X 4	2 X 4	00-00-00 00-00-00	01-06-08 08-01-04	231.32 146.34		
	2	T45A HALF HIP	8.00 0.00	24-03-00	09-01-04	2 X 4	2 X 4	00-00-00 00-00-00	01-06-08 09-01-04	242.42 150.34		
	3	T46A HALF HIP	8.00 0.00	24-03-00	10-01-04	2 X 4	2 X 4	00-00-00 00-00-00	01-06-08 10-01-04	381.24 237.00		
	1	T47 FLAT GIRDER	0.00 0.00	24-03-00	04-01-04	2 X 6	2 X 6	00-00-00 00-00-00	04-01-04 04-01-04	145.78 90.67		
	1	T48 COMMON	8.00 0.00	14-09-00	06-03-13	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	63.33 39.00		
	2	T48A COMMON	8.00 0.00	14-06-08	06-03-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	122.14 76.66		
	3	T48AA COMMON	8.00 0.00	14-06-08	06-03-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	183.21 114.99		
	2	T610 ROOF	8.00 8.00	14-09-00	06-03-13	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	125.70 83.66		
	2	T800 COMMON	6.50 0.00	07-04-00	02-07-04	2 X 4	2 X 4	01-03-08 01-03-08	00-07-07 00-07-07	48.84 31.34		
	1	T800Z COMMON	6.50 0.00	07-04-00	02-07-04	2 X 4	2 X 4	01-03-08 01-03-08	00-07-07 00-07-07	24.42 15.67		
	17	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	285.43 181.39		
	3	J88 JACK-OPEN	8.00 0.00	02-08-08	02-07-05	2 X 4	2 X 4	01-03-08 00-00-00	00-09-10 02-07-05	27.57 18.00		
	9	J93 MONOPITCH	4.00 0.00	06-05-00	02-05-10	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-05-10	163.44 105.03		

TOTAL # TRUSS= 55

TOTAL BFT OF ALL TRUSSES=

1770.76 BFT. TOTAL WEIGHT OF ALL TRUSSES= 2808.90 LBS.



Delivery Shiplist

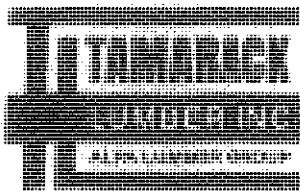
DATE	07/03/18
SALES REP	Mario

JOB TRACK: 43954	LAYOUT ID: 285485	LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH	SUB-BUILDER:	
MODEL: HIGHGROVE 4	ELEVATION: 3- UNIT2	

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	HGUS26-2	
3	Hangers	LJS26DS	

TOTAL # ITEMS= 4



Delivery Shiplist

DATE	06/26/18
SALES REP	Mario

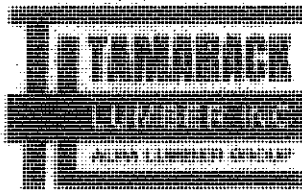
Page 1 of 2

JOB TRACK: 43954	LAYOUT ID: 285492	LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH	SUB-BUILDER:	
MODEL: HIGHGROVE 2	ELEVATION: 3 (REV)-UNIT3	

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	3	T71X COMMON	6.00 0.00	31-00-00	08-08-12	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 00-00-00	368.61 229.50		
	10	T90TCX MONOPITCH	3.00 0.00	10-04-00	03-05-09	2 X 6	2 X 4	00-09-08 00-00-00	00-08-04 03-05-09	449.80 303.30		
	1	G90 MONOPITCH	3.00 0.00	10-04-00	03-03-04	2 X 4	2 X 4	00-09-08 00-00-00	00-08-04 03-03-04	59.08 38.33		
	2	T92 COMMON	6.50 0.00	08-03-00	02-03-12	2 X 4	2 X 4	01-03-08 01-03-08	00-07-07 00-07-07	43.08 30.00		
	1	T92Z4 COMMON	6.50 0.00	08-03-00	02-03-12	2 X 4	2 X 4	01-03-08 01-03-08	00-07-07 00-07-07	21.54 15.00		
	1	T620 HIP GIRDER	6.00 0.00	36-04-00	05-03-13	2 X 4	2 X 6	01-03-08 01-03-08	01-02-00 01-02-00	213.66 131.66		
	1	T630 HIP	6.00 0.00	36-04-00	06-07-13	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	146.04 92.83		
	1	T640 HIP	6.00 0.00	36-04-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	246.52 153.33		
	1	T650 HIP	6.00 0.00	36-04-00	09-03-13	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	156.18 98.00		
	2	T660 COMMON	6.00 0.00	36-04-00	10-03-00	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	293.96 180.00		
	1	T670S ROOF	6.00 0.00	36-04-00	10-03-00	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-04-08	161.65 100.67		
	1	T680S ROOF	6.00 0.00	36-04-00	10-03-00	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-04-08	173.83 108.67		
	1	T690S ROOF	6.00 0.00	36-04-00	10-01-13	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-04-08	176.48 108.83		
	1	T700 ROOF	6.00 0.00	36-04-00	11-03-02	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-04-08	188.58 116.33		
	1 3 Ply	T720 HALF HIP	8.00 0.00	13-11-00	04-00-12	2 X 4	2 X 6	00-00-00 00-00-00	02-04-13 00-00-04	247.65 161.01		
	2	J10 JACK-OPEN	8.00 0.00	06-10-08	05-03-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 00-05-03	35.66 21.34		
	1	J81 JACK-OPEN	6.50 0.00	03-01-08	02-03-12	2 X 4	2 X 4	01-03-08 00-00-00	00-07-07 02-03-12	10.08 7.33		
	2	J82 JACK-OPEN	6.50 0.00	03-01-08	01-07-01	2 X 4	2 X 4	01-03-08 -01-04-01	00-07-07 00-03-08	16.70 12.00		



Delivery Shiplist

DATE	06/26/18
SALES REP	Mario

Page 2 of 2

JOB TRACK: 43954	LAYOUT ID: 285492	LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH	SUB-BUILDER:	
MODEL: HIGHGROVE 2	ELEVATION: 3 (REV)-UNIT3	

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	2	J83 JACK-OPEN	6.50 0.00	01-10-08	01-07-01	2 X 4	2 X 4	01-03-08 -00-01-01	00-07-07 00-03-08	13.84 9.34		
	5	J93 MONOPITCH	4.00 0.00	06-05-00	02-05-10	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-05-10	90.80 58.35		
	7	J110 JACK-OPEN	10.00 0.00	01-08-08	04-00-12	2 X 4	2 X 4	01-03-08 00-00-00	02-07-11 04-00-12	64.40 46.69		

TOTAL # TRUSS= 48

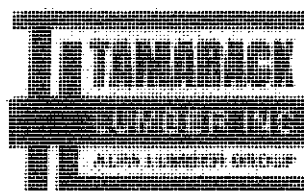
TOTAL BFT OF ALL TRUSSES=

2023.51 BFT. TOTAL WEIGHT OF ALL TRUSSES= 3180.22 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
10	Hangers	LJS26DS	
1	Hangers	HGUS26-2	

TOTAL # ITEMS= 11



Delivery Shiplist

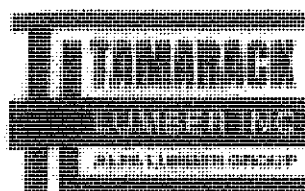
DATE	08/26/18
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 285494 LOCATION:
 BUILDER: GREENPARK - RUSSELL GARDENS PH SUB-BUILDER:
 MODEL: HIGHGROVE2 ELEVATION: 3-UNIT4 OR 3-UNIT 3,4

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	3	T24 COMMON	6.00 0.00	31-00-00	08-11-00	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	364.65 229.50		
	9	T90TCX MONOPITCH	3.00 0.00	10-04-00	03-05-09	2 X 6	2 X 4	00-09-08 00-00-00	00-08-04 03-05-09	404.91 272.97		
	2	G90 MONOPITCH	3.00 0.00	10-04-00	03-03-04	2 X 6	2 X 4	00-09-08 00-00-00	00-08-04 03-03-04	81.80 53.34		
	2	T92 COMMON	6.50 0.00	08-03-00	02-03-12	2 X 4	2 X 4	01-03-08 01-03-08	00-07-07 00-07-07	43.08 30.00		
	1	T92Z4 COMMON	6.50 0.00	08-03-00	02-03-12	2 X 4	2 X 4	01-03-08 01-03-08	00-07-07 00-07-07	21.54 15.00		
	1	T620Z HIP GIRDER	6.00 0.00	36-04-00	05-03-13	2 X 4	2 X 6	01-03-08 01-03-08	01-02-00 01-02-00	213.66 131.66		
	1	T630 HIP	6.00 0.00	36-04-00	06-07-13	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	146.04 92.83		
	1	T640 HIP	6.00 0.00	36-04-00	07-11-13	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	148.64 93.00		
	1	T650 HIP	6.00 0.00	36-04-00	09-03-13	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	156.18 98.00		
	2	T660 COMMON	6.00 0.00	36-04-00	10-03-00	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	293.96 180.00		
	1	T670S ROOF	6.00 0.00	36-04-00	10-03-00	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-04-08	161.65 100.67		
	1	T680S ROOF	6.00 0.00	36-04-00	10-03-00	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-04-08	173.83 108.67		
	1	T690S ROOF	6.00 0.00	36-04-00	10-01-13	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-04-08	178.48 109.83		
	1	T700 ROOF	6.00 0.00	36-04-00	11-03-02	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-04-08	188.58 116.33		
	1 3 Ply	T720 HALF HIP	8.00 0.00	13-11-00	04-00-12	2 X 4	2 X 6	00-00-00 00-00-00	02-04-13 00-00-04	247.65 161.01		
	2	J10 JACK-OPEN	8.00 0.00	06-10-08	05-03-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 00-05-03	35.66 21.34		
	1	J81 JACK-OPEN	6.50 0.00	03-01-08	02-03-12	2 X 4	2 X 4	01-03-08 00-00-00	00-07-07 02-03-12	10.08 7.33		
	2	J82 JACK-OPEN	6.50 0.00	03-01-08	01-07-01	2 X 4	2 X 4	01-03-08 -01-04-01	00-07-07 00-03-08	16.70 12.00		



Delivery Shiplist

Page 2 of 2

DATE	08/26/18
SALES REP	Mano

JOB TRACK: 43954	LAYOUT ID: 285494	LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH	SUB-BUILDER:	
MODEL: HIGHGROVE2	ELEVATION: 3-UNIT4 OR 3-UNIT 3,4	

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOY					
	2	J83 JACK-OPEN	6.50 0.00	01-10-08	01-07-01	2 X 4	2 X 4	01-03-08 -00-01-01	00-07-07 00-03-08	13.84 9.34		
	5	J93 MONOPITCH	4.00 0.00	08-06-00	02-05-10	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-05-10	90.80 58.35		
	7	J110 JACK-OPEN	10.00 0.00	01-08-08	04-00-12	2 X 4	2 X 4	01-03-08 00-00-00	02-07-11 04-00-12	64.40 46.69		

TOTAL # TRUSS= 49

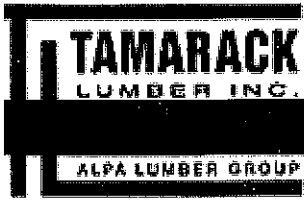
TOTAL BFT OF ALL TRUSSES=

1947.86 BFT. TOTAL WEIGHT OF ALL TRUSSES= 3056.13 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-10
10	Hangers	LJS26DS	
1	Hangers	HGUS26-2	

TOTAL # ITEMS= 11



Delivery Shiplist

DATE	09/07/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 285864 D LOCATION:
 BUILDER: GREENPARK - RUSSELL GARDENS PH2 SUB-BUILDER:
 MODEL: HIGHGROVE 3 ELEVATION: 3 (REV)-UNIT5

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	7	T24 COMMON	6.00 0.00	31-00-00	08-11-00	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	850.85 535.50		
	1	T73S ROOF	6.00 0.00	31-00-00	06-01-02	2 X 4	2 X 4	00-00-00 00-00-00	05-02-08 04-11-08	141.76 90.33		
	1	T74S ROOF	6.00 0.00	31-00-00	07-05-02	2 X 4	2 X 4	00-00-00 00-00-00	06-06-08 04-11-08	158.68 99.33		
	1	T179 HIP	6.00 0.00	31-00-00	07-10-08	2 X 4	2 X 4	00-00-00 00-00-00	06-11-00 02-07-00	148.46 94.17		
	1	T180 HIP	6.00 0.00	31-00-00	09-02-08	2 X 4	2 X 4	00-00-00 00-00-00	06-11-00 02-07-00	151.53 94.50		
	1	T181 HIP	6.00 0.00	31-00-00	10-06-08	2 X 4	2 X 4	00-00-00 00-00-00	06-11-00 02-07-00	158.15 98.17		
	1	T182 HIP	6.00 0.00	31-00-00	11-03-13	2 X 4	2 X 4	00-00-00 00-00-00	06-11-00 02-07-00	162.00 100.67		
	1	T183S ROOF	8.00 0.00	17-01-08	06-00-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 03-06-08	76.28 49.33		
	1 3 Ply	T184 HIP GIRDER	8.00 0.00	17-01-08	04-08-08	2 X 6	2 X 6	00-00-00 00-00-00	01-04-13 03-06-08	323.25 202.02		
	1	T30 HALF HIP	8.00 0.00	15-09-08	04-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-04-13 04-01-04	77.31 47.50		
	1	T31 HALF HIP	8.00 0.00	15-09-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-01-04	67.94 43.00		
	1	T32 HALF HIP	8.00 0.00	15-09-08	06-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 06-01-04	74.63 46.67		
	1 3 Ply	T33Z4 HALF HIP	8.00 0.00	15-09-08	06-08-12	2 X 6	2 X 6	00-00-00 00-00-00	01-04-13 06-08-12	341.19 201.00		
	2	T102 HALF HIP	10.00 0.00	02-11-08	02-01-08	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 04-01-08	35.80 25.00		
	2	T103 HALF HIP	10.00 0.00	02-11-08	03-05-08	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 05-05-08	42.06 28.66		
	2	T93 COMMON	6.50 0.00	06-06-00	02-04-09	2 X 4	2 X 4	01-03-08 01-03-08	00-07-07 00-07-07	44.40 30.00		
	1	T93Z COMMON	6.50 0.00	06-06-00	02-04-09	2 X 4	2 X 4	01-03-08 01-03-08	00-07-07 00-07-07	22.20 15.00		
	8	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	134.32 85.36		



Delivery Shiplist

DATE	09/07/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 285864 LOCATION:
 BUILDER: GREENPARK - RUSSELL GARDENS PH1 SUB-BUILDER:
 MODEL: HIGHGROVE 3 ELEVATION: 3 (REV)-UNIT5

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	5	J12 JACK-OPEN	8.00 0.00	02-11-08	03-04-08	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 00-05-03	52.55 33.35		
	3	J80 JACK-OPEN	8.00 0.00	02-04-04	02-04-07	2 X 4	2 X 4	01-03-08 00-00-00	00-09-10 02-04-07	24.93 18.00		
	4	J93 MONOPITCH	4.00 0.00	06-05-00	02-05-10	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-05-10	72.64 46.68		
	4	J99 MONOPITCH	4.00 0.00	05-09-00	02-02-15	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-02-15	60.52 37.32		

TOTAL # TRUSS= 54.00

TOTAL BFT OF ALL TRUSSES=

2021.56 BFT. TOTAL WEIGHT OF ALL TRUSSES= 3221.45 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
4	Hangers	LUS24	
17	Hangers	LJS26DS	

TOTAL # ITEMS= 21.00



Delivery Shiplist

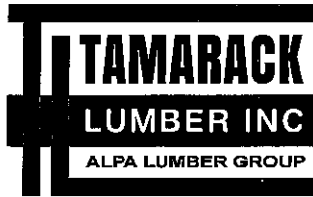
DATE	04/18/18
SALES REP	Mario

JOB TRACK: 43954	LAYOUT ID: 285865	LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH2	SUB-BUILDER:	
MODEL: HIGHGROVE 5E	ELEVATION: 3 (REV)-UNIT6	

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY PLY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
						TOP	BOT					
	1	T8Z HALF HIP	8.00 0.00	24-03-08	04-01-04	2 X 6	2 X 6	01-03-08 00-00-00	01-04-13 04-01-04	143.26 88.34		
	1	T8Z4 HALF HIP	8.00 0.00	24-03-08	04-01-04	2 X 6	2 X 6	01-03-08 00-00-00	01-04-13 04-01-04	143.26 88.34		
	2	T9 HALF HIP	8.00 0.00	24-03-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-01-04	197.10 123.66		
	2	T10 HALF HIP	8.00 0.00	24-03-08	06-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 06-01-04	212.66 133.34		
	2	T11 HALF HIP	8.00 0.00	24-03-08	07-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 07-01-04	225.56 142.68		
	2	T12 HALF HIP	8.00 0.00	24-03-08	08-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 08-01-04	234.64 146.34		
	6	T13 HALF HIP	8.00 0.00	24-03-08	09-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 09-01-04	737.28 454.98		
	2	T37 HALF HIP	8.00 0.00	24-03-08	10-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 10-01-04	257.54 159.34		
	2	T15A COMMON	8.00 0.00	15-05-08	06-07-08	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	129.00 82.66		
	1	G15A COMMON	8.00 0.00	15-05-08	06-07-08	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	67.52 44.33		
	2	T14 COMMON	8.00 0.00	14-04-00	06-02-02	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	123.52 78.00		
	1	T570 HIP GIRDER	8.00 0.00	14-04-00	03-11-13	2 X 4	2 X 6	01-03-08 01-03-08	01-04-13 01-04-13	70.63 46.34		
	1	T580 HIP	8.00 0.00	14-04-00	05-03-13	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	62.73 39.33		
	2	T590 ROOF	8.00 8.00	08-08-00	04-03-08	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	83.72 58.68		
	1	T600 COMMON	8.00 0.00	08-08-00	04-03-08	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	37.48 23.67		
	3	T91 COMMON	6.50 0.00	06-06-00	02-04-09	2 X 4	2 X 4	01-03-08 01-03-08	00-07-07 00-07-07	66.60 45.00		
	1	G91 COMMON	6.50 0.00	06-06-00	02-04-09	2 X 4	2 X 4	01-03-08 01-03-08	00-07-07 00-07-07	23.39 16.00		
	2	P6 PIGGYBACK	8.00 0.00	11-11-15	01-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 01-10-08	80.22 51.34		



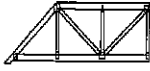
Delivery Shiplist

DATE	04/18/18
SALES REP	Mario

JOB TRACK: 43954	LAYOUT ID: 285865	LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH2	SUB-BUILDER:	
MODEL: HIGHGROVE 5E	ELEVATION: 3 (REV)-UNIT6	

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	2	P7 PIGGYBACK	8.00 0.00	11-11-15	02-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 02-10-08	85.52 56.00		
	8	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	134.32 85.36		
	1	J8 JACK-OPEN	8.00 0.00	03-10-08	03-11-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 03-11-13	12.81 8.00		
	1	J20 JACK-OPEN	8.00 0.00	01-10-08	02-07-02	2 X 4	2 X 4	01-03-08 -00-01-01	01-04-13 00-03-08	9.17 5.67		
	1	J21 JACK-OPEN	8.00 0.00	03-10-08	02-07-02	2 X 4	2 X 4	01-03-08 -02-01-01	01-04-13 00-03-08	12.97 8.00		
	4	J90 JACK	4.50 0.00	05-05-00	02-05-00	2 X 4	2 X 4	01-03-08 00-00-00	00-04-10 02-05-00	57.88 37.32		

TOTAL # TRUSS= 51.00

TOTAL BFT OF ALL TRUSSES=

2022.72 BFT. TOTAL WEIGHT OF ALL TRUSSES= 3208.78 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	HGUS26-2	
5	Hangers	LJS26DS	
2	Hangers	LUS24	

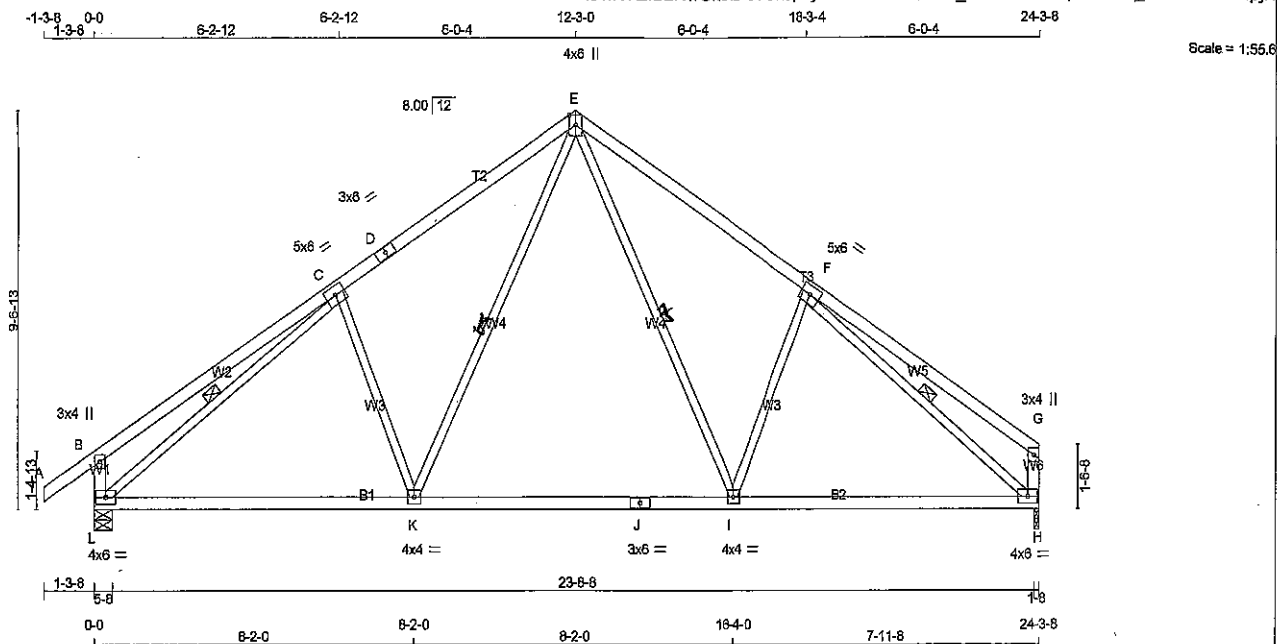
TOTAL # ITEMS= 10.00

JOB NAME 285994	TRUSS NAME T1A	QUANTITY 1	PLY 1	JOB DESC. 43954	DRWG NO.
TRUSS DESC.					

Tamarack Roof Truss, Burlington

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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4 DRY No.2	SPF			
D - E	2x4 DRY No.2	SPF			
E - G	2x4 DRY No.2	SPF			
L - B	2x4 DRY No.2	SPF			
H - G	2x4 DRY No.2	SPF			
L - J	2x4 DRY No.2	SPF			
J - H	2x4 DRY No.2	SPF			

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW+1	MT20	5.0	6.0		
D	TS-t	MT20	3.0	6.0		
E	TTWW+p	MT20	4.0	6.0	Edge	
F	TMVW+1	MT20	5.0	6.0		
G	TMV+p	MT20	3.0	4.0		
H	BMVW+1-t	MT20	4.0	6.0		
I	BMVW+1	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMVW+1	MT20	4.0	4.0		
L	BMVW+1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
		GROSS REACTION		GROSS REACTION		BRG		BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX		
L	1479	0	1479	0	0	5-8	5-8		
H	1363	0	1363	0	0	1-8	1-8		

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1198	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0
H	1119	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.95 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-L, F-H. *E74, E71*

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM	TO		FR-TO		
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	C-K	-352 / 44 0.19 (1)
B-C	0 / 34	-84.3	-84.3	0.52 (1)	10.00	K-E	0 / 632 0.14 (1)
C-D	-1409 / 0	-84.3	-84.3	0.45 (1)	4.96	E-I	0 / 599 0.13 (1)
D-E	-1409 / 0	-84.3	-84.3	0.45 (1)	4.96	I-F	-315 / 58 0.17 (1)
E-F	-1388 / 0	-84.3	-84.3	0.44 (1)	5.01	L-C	-1889 / 0 0.71 (1)
F-G	0 / 34	-84.3	-84.3	0.50 (1)	10.00	F-H	-1853 / 0 0.67 (1)
L-B	-311 / 0	0.0	0.0	0.03 (1)	7.81		
H-G	-186 / 0	0.0	0.0	0.02 (1)	7.81		
L-K	0 / 1275	-28.0	-28.0	0.58 (2)	10.00		
K-J	0 / 889	-28.0	-28.0	0.54 (2)	10.00		
J-I	0 / 889	-28.0	-28.0	0.54 (2)	10.00		
I-H	0 / 1245	-28.0	-28.0	0.58 (2)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC0210

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/999 (0.17")
ALLOWABLE DEFL.(TL) = L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/999 (0.29")

CSI: TC=0.52/1.00 (B-C:1), BC=0.58/1.00 (K-L:2), WB=0.71/1.00 (C-L:1), SS=0.21/1.00 (C-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

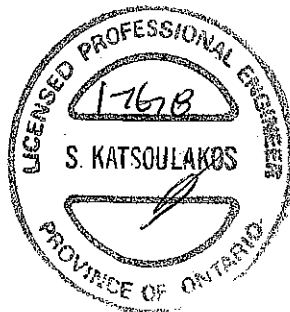
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	618	354	1687
	822	2284	1656

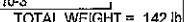
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

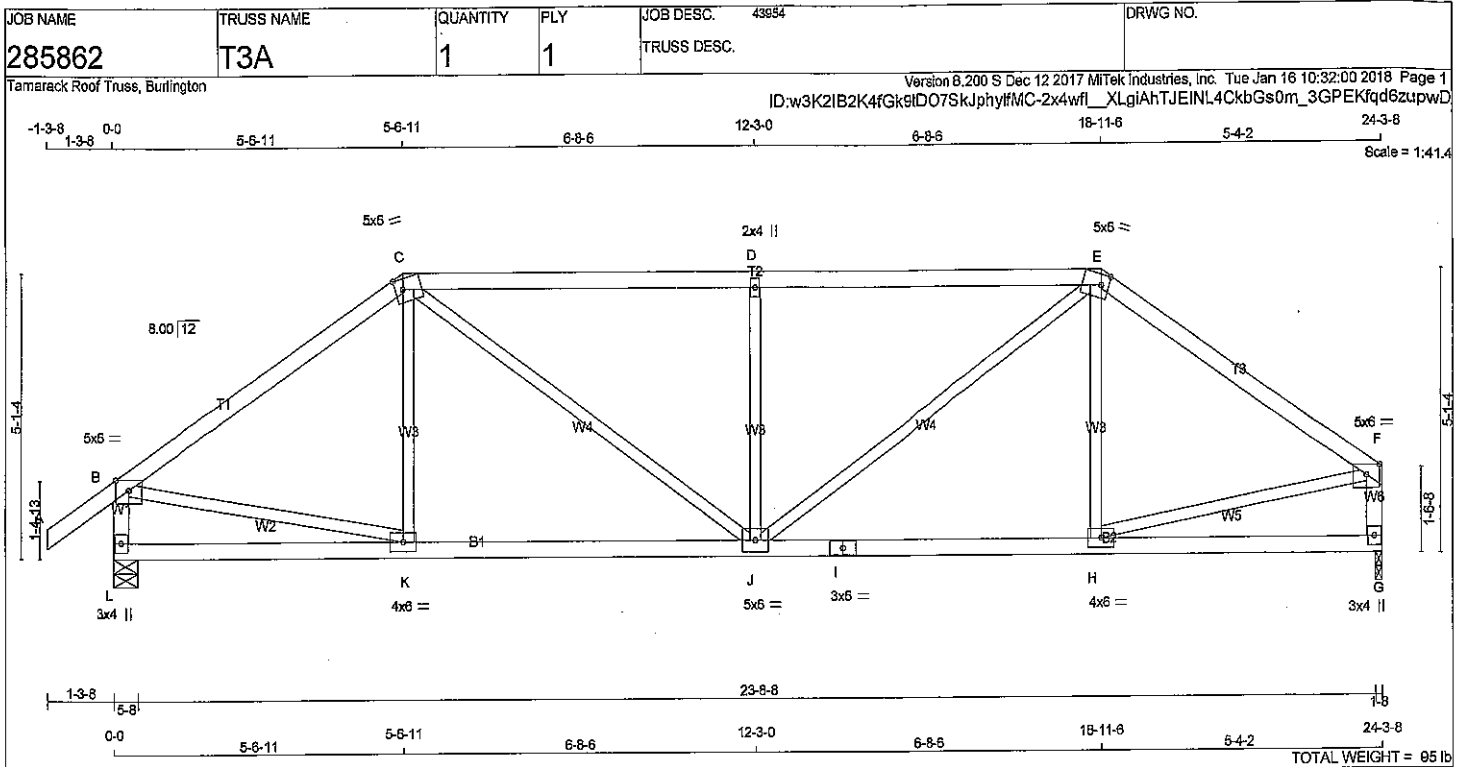
JSI GRIP= 0.89 (L) (INPUT = 0.90)
JSI METAL= 0.43 (C) (INPUT = 1.00)



DRWG NO. **TAM3984-18**
STRUCTURAL
COMPONENT ONLY



DWG NO. TAM
STRUCTURAL
COMPONENT ONLY



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
E - F	2x4	DRY	No.2	SPF	
L - B	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
L - I	2x4	DRY	No.2	SPF	
I - G	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge		
C	TTWW-m	MT20	5.0	6.0	2.25	1.75	
D	TMVW-w	MT20	2.0	4.0			
E	TTWW-m	MT20	5.0	6.0	2.25	1.75	
F	TMVW-p	MT20	5.0	6.0	Edge		
G	BMV1+p	MT20	3.0	4.0			
H	BMVW-t	MT20	4.0	8.0			
I	BS-t	MT20	3.0	6.0			
J	BMVW-w-t	MT20	5.0	6.0			
K	BMVW-t	MT20	4.0	6.0			
L	BMV1+p	MT20	3.0	4.0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

<u>BEARINGS</u>							
	FACTORED		MAXIMUM FACTORED			INPUT	REQ'D
	GROSS REACTION		GROSS REACTION			BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
L	1479	0	1479	0	0	5-8	5-8
G	1363	0	1363	0	0	1-8	1-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1198	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0
G	1118	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX. CS1 (LC)	UNBRAC LENGTH	FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	K-C	-45 / 211	0.05 (3)	
B-C	-1495 / 0	-84.3	-84.3	0.55 (1)	4.69	C-J	0 / 728	0.16 (1)	
C-D	-1833 / 0	-84.3	-84.3	0.80 (1)	3.88	J-D	-662 / 0	0.27 (1)	
D-E	-1833 / 0	-84.3	-84.3	0.80 (1)	3.88	J-E	0 / 769	0.17 (1)	
E-F	-1456 / 0	-84.3	-84.3	0.50 (1)	4.81	H-E	-51 / 187	0.04 (3)	
L-B	-1418 / 0	0.0	0.0	0.15 (1)	6.85	B-K	0 / 1265	0.28 (1)	
G-F	-1305 / 0	0.0	0.0	0.14 (1)	7.08	H-F	0 / 1239	0.28 (1)	
L-K	0 / 0	-28.0	-28.0	0.25 (3)	10.00				
K-J	0 / 1241	-28.0	-28.0	0.40 (2)	10.00				
J-I	0 / 1208	-28.0	-28.0	0.39 (2)	10.00				
I-H	0 / 1208	-28.0	-28.0	0.39 (2)	10.00				
H-G	0 / 0	-28.0	-28.0	0.24 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.8	PSF
	DL = 3.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 089-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/999 (0.14")

CSI: TC=0.80/1.00 (D-E:1), BC=0.40/1.00 (J-K:2), WB=0.28/1.00 (B-K:1), SSI=0.27/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

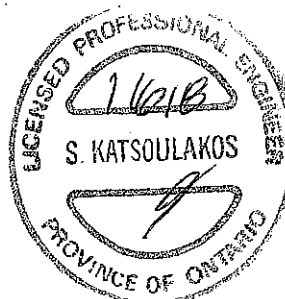
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1867 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (E) (INPUT = 0.90)
JSI METAL= 0.36 (I) (INPUT = 1.00)



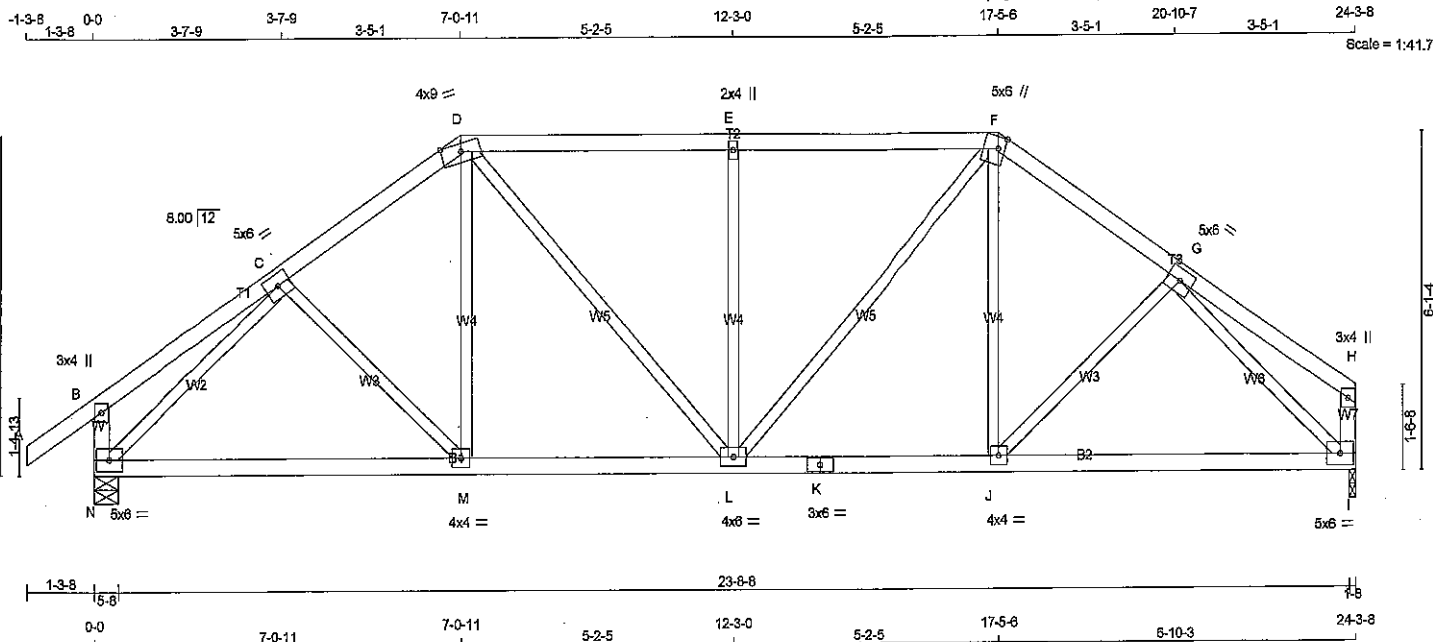
DWG NO. TAM 4008-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43854	DRWG NO.
285862	T4A	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:w3K2IB2K4fGk9ID07SkJphyfIMC-2x4wfl_XLgAhTJEINL4Ckjs7l_zhPEKfqd6zupwd



<u>LUMBER</u>				
N. L. G. A. RULES				
CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0			
C	TMVW-t	MT20	5.0	6.0			
D	TTWW-m	MT20	4.0	9.0	Edge		
E	TMV+w	MT20	2.0	4.0			
F	TTWW+m	MT20	5.0	6.0	2.50	1.50	
G	TMVW-t	MT20	5.0	6.0			
H	TMV+p	MT20	3.0	4.0			
I	BMVW-t	MT20	5.0	6.0			
J	BMVW-t	MT20	4.0	4.0			
K	BS-t	MT20	3.0	6.0			
L	BMVW-t	MT20	4.0	6.0			
M	BMVW-t	MT20	4.0	4.0			
N	BMVW-t	MT20	5.0	6.0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQ'D
	GROSS REACTION	GROSS REACTION	BRG	BRG
	VERT	HORZ	DOWN	HORZ
JT	VERT	HORZ	UPLIFT	IN-SX
N	1479	0	0	5-8
I	1363	0	0	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX	MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE	PERM. LIVE
N	1198	692 / 0	255 / 0	0 / 0
I	1119	621 / 0	255 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.05 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)
FR-TO									
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	C-M	0 / 117	0.03 (3)	
B-C	0 / 17	-84.3	-84.3	0.15 (1)	10.00	M-D	0 / 305	0.07 (2)	
C-D	-1458 / 0	-84.3	-84.3	0.14 (1)	5.29	D-L	0 / 409	0.09 (1)	
D-E	-1474 / 0	-84.3	-84.3	0.31 (1)	5.05	L-E	-534 / 0	0.31 (1)	
E-F	-1474 / 0	-84.3	-84.3	0.31 (1)	5.05	F-F	0 / 442	0.10 (1)	
F-G	-1433 / 0	-84.3	-84.3	0.14 (1)	5.33	J-F	0 / 273	0.06 (2)	
G-H	0 / 17	-84.3	-84.3	0.14 (1)	10.00	J-G	0 / 138	0.03 (3)	
N-B	-233 / 0	0.0	0.0	0.02 (1)	7.81	N-C	-1669 / 0	0.64 (1)	
I-H	-108 / 0	0.0	0.0	0.01 (1)	7.81	G-I	-1635 / 0	0.59 (1)	
N-M	0 / 1198	-28.0	-28.0	0.45 (2)	10.00				
M-L	0 / 1201	-28.0	-28.0	0.46 (2)	10.00				
L-K	0 / 1179	-28.0	-28.0	0.45 (2)	10.00				
K-J	0 / 1179	-28.0	-28.0	0.45 (2)	10.00				
J-I	0 / 1136	-28.0	-28.0	0.43 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.8	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD	=	46.1	PSF	

SPACING = 24.0 IN. GIG

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL) = L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/999 (0.18")

CSI: TC=0.31/1.00 (D-E:1), BC=0.46/1.00 (L-M:2), WB=0.64/1.00 (C-N:1), SS=0.21/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

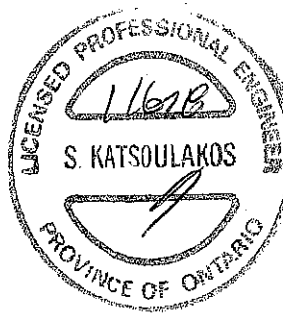
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP (DRY) SHEAR (PSI)	SECTION (PLI)
MT20	618 354 1667 822 2284 1656	

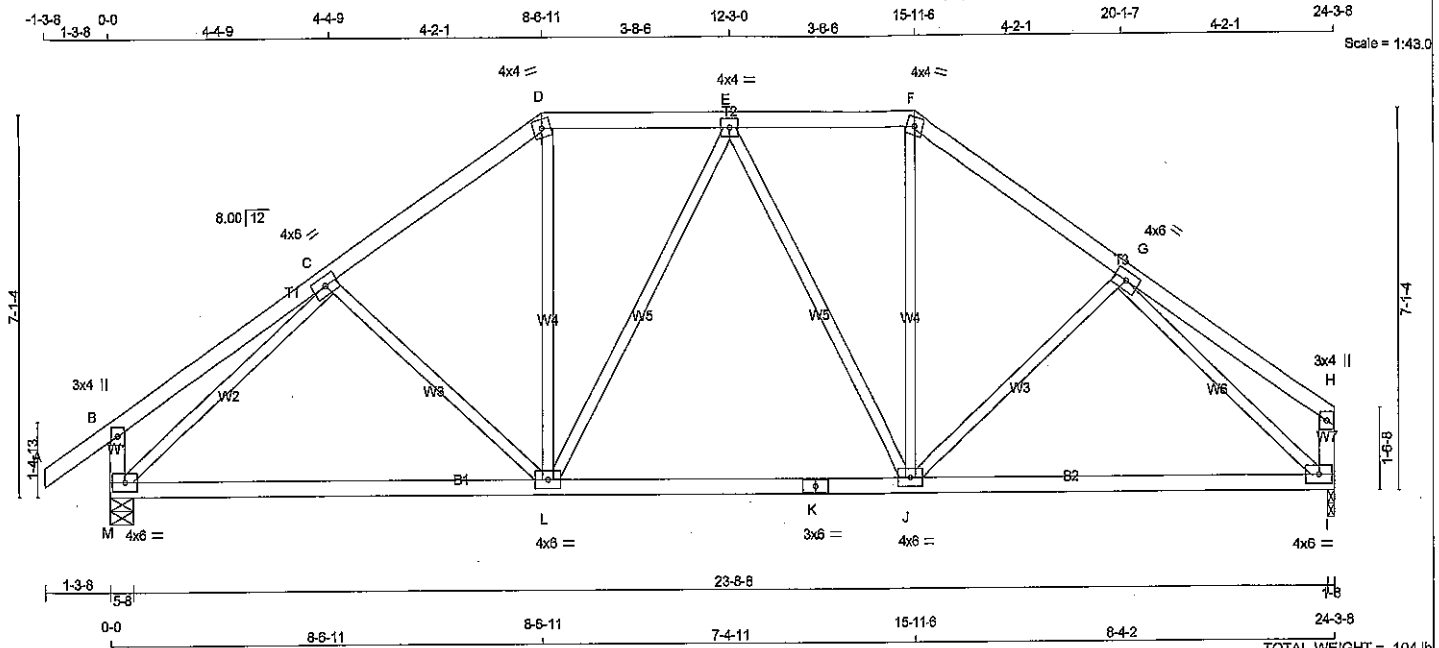
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (D) (INPUT = 0.80)
JSI METAL= 0.41 (C) (INPUT = 1.00)



DRWG NO. TAM 4009-18
STRUCTURAL
COMPONENT ONLY



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
D - F	2x4	DRY No.2	SPF
F - H	2x4	DRY No.2	SPF
M - B	2x4	DRY No.2	SPF
I - H	2x4	DRY No.2	SPF
M - K	2x4	DRY No.2	SPF
K - I	2x4	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	4.0		
G	TMWW-t	MT20	4.0	6.0		
H	TMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	6.0		
J	BMVWW-t	MT20	4.0	6.0		
K	BS-t	MT20	3.0	6.0		
L	BMVWW-t	MT20	4.0	6.0		
M	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
M	1479	0	1479	0	5-8	5-8
I	1363	0	1363	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN COMPONENT REACTIONS		DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE		
M	1198	692 / 0	255 / 0	0 / 0	251 / 0	0 / 0
I	1119	621 / 0	255 / 0	0 / 0	243 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.32 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		MAX. UNBRAC	MAX. FACTORED
	MAX. FORCE (LBS)	VERT. LOAD (PLF)	VERT. LOAD (PLF)	MAX. (LBS)		
FR-TO						
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	C-L -117 / 85 0.07 (1)
B-C	0 / 22	-84.3	-84.3	0.24 (1)	10.00	L-D 0 / 495 0.11 (1)
C-D	-1388 / 0	-84.3	-84.3	0.21 (1)	5.32	L-E -195 / 0 0.24 (1)
D-E	-1142 / 0	-84.3	-84.3	0.15 (1)	5.79	E-J -225 / 0 0.27 (1)
E-F	-1128 / 0	-84.3	-84.3	0.15 (1)	5.82	J-F 0 / 484 0.11 (1)
F-G	-1371 / 0	-84.3	-84.3	0.20 (1)	5.35	J-G -70 / 102 0.04 (1)
G-H	0 / 22	-84.3	-84.3	0.23 (1)	10.00	M-C -1660 / 0 0.93 (1)
M-B	-255 / 0	0.0	0.0	0.03 (1)	7.81	G-I -1632 / 0 0.85 (1)
I-H	-131 / 0	0.0	0.0	0.01 (1)	7.91	
M-L	0 / 1222	-28.0	-28.0	0.57 (2)	10.00	
L-K	0 / 1233	-28.0	-28.0	0.58 (2)	10.00	
K-J	0 / 1233	-28.0	-28.0	0.58 (2)	10.00	
J-I	0 / 1173	-28.0	-28.0	0.54 (2)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	PSF
LL	25.6	PSF
BOT CH.	DL	PSF
DL	10.5	PSF
DL	7.0	PSF
TOTAL LOAD	40.1	PSF

SPACING = 24.0 IN C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL)= L/999 (0.23")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL)= L/761 (0.38")

CSI: TC=0.24/1.00 (B-C:1), BC=0.58/1.00 (J-L:2),
WB=0.93/1.00 (C-M:1), SS=0.19/1.00 (L-M:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1867
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (M) (INPUT = 0.90)
JSI METAL= 0.40 (C) (INPUT = 1.00)



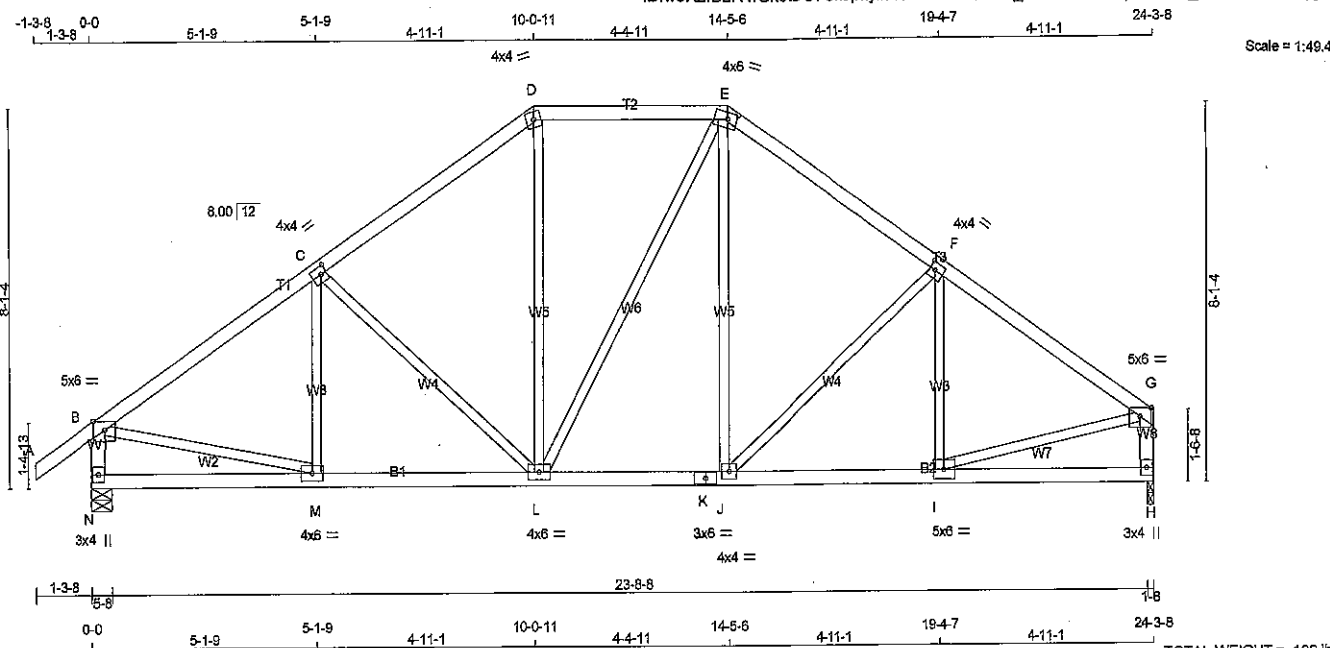
DRWG NO. TAM 3985-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
285994	T6A	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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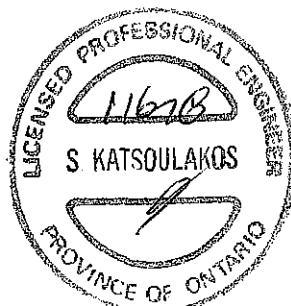
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Scale = 1:49.4

TOTAL WEIGHT = 109 lb

LUMBER N. L. G. A. RULES CHORDS SIZE A - D 2x4 DRY No.2 SPF D - E 2x4 DRY No.2 SPF E - G 2x4 DRY No.2 SPF N - B 2x4 DRY No.2 SPF H - G 2x4 DRY No.2 SPF N - K 2x4 DRY No.2 SPF K - H 2x4 DRY No.2 SPF ALL WEBS 2x3 DRY No.2 SPF EXCEPT DRY: SEASONED LUMBER.				DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 25.8 PSF DL = 3.0 PSF BOT CH. LL = 10.5 PSF DL = 7.0 PSF TOTAL LOAD = 46.1 PSF SPACING = 24.0 IN. C/C LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12 THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010 THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012, BCBC 2012, ABC 2014 - CSA 088-09 - TPIC 2011 (55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL)= L/360 (0.81") CALCULATED VERT. DEFL.(LL) = L/999 (0.05") ALLOWABLE DEFL.(TL)= L/360 (0.81") CALCULATED VERT. DEFL.(TL) = L/999 (0.08") CSI: TC=0.31/1.00 (B-C:1), BC=0.32/1.00 (L-M:2), WB=0.30/1.00 (C-L:1), SSI=0.17/1.00 (B-C:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10 COMPANION LIVE LOAD FACTOR = 0.50 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (P.S.I) (P.L.I) (P.L.I) MAX MIN MAX MIN MAX MIN MT20 818 354 1687 822 2284 1658 PLATE PLACEMENT TOL = 0.250 inches PLATE ROTATION TOL = 5.0 Deg. JSI GRIP= 0.89 (M) (INPUT = 0.90) JSI METAL= 0.32 (M) (INPUT = 1.00)			
PLATES (table is in inches) JT TYPE PLATES W LEN Y X B TMVW-p MT20 5.0 6.0 Edge C TMWW-t MT20 4.0 4.0 2.00 1.50 D TTW-m MT20 4.0 4.0 E TTWW-m MT20 4.0 6.0 1.75 2.50 F TMWW-t MT20 4.0 4.0 2.00 1.50 G TMVW-p MT20 5.0 6.0 Edge H BMV1+p MT20 3.0 4.0 I BMWW-t MT20 5.0 6.0 J BMWW-t MT20 4.0 4.0 K SS-t MT20 3.0 6.0 L BMWW-t MT20 4.0 6.0 M BMWW-t MT20 4.0 6.0 N BMV1+p MT20 3.0 4.0 Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.				BEARINGS FACTORED GROSS REACTION JT VERT HORZ N 1479 0 H 1363 0 MAXIMUM FACTORED GROSS REACTION DOWN HORZ UPLIFT IN-SX N 1479 0 0 5-8 H 1363 0 0 1-8 REQD BRG IN-SX N 5-8 H 1-8			
UNFACTORED REACTIONS 1ST LCASE MAX/MIN COMPONENT REACTIONS JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL N 1198 692 / 0 255 / 0 0 / 0 0 / 0 251 / 0 0 / 0 H 1118 621 / 0 255 / 0 0 / 0 0 / 0 243 / 0 0 / 0				LOADING TOTAL LOAD CASES: (4) CHORDS MEMB. MAX. FACTORED FORCE (LBS) VERT. LOAD LC1 MAX (PLF) CSI (LC) UNBRAC LENGTH FR-TO A-B 0 / 32 -84.3 -84.3 0.11 (1) 10.00 M-C -115 / 147 0.04 (1) B-C -1531 / 0 -84.3 -84.3 0.31 (1) 5.00 C-L -354 / 0 0.30 (1) C-D -1271 / 0 -84.3 -84.3 0.29 (1) 5.39 L-D 0 / 374 0.08 (2) D-E -1037 / 0 -84.3 -84.3 0.22 (1) 5.91 L-E 0 / 15 0.00 (1) E-F -1262 / 0 -84.3 -84.3 0.28 (1) 5.42 J-E 0 / 360 0.08 (2) F-G -1483 / 0 -84.3 -84.3 0.29 (1) 5.08 J-F -310 / 0 0.26 (1) G-H -1420 / 0 0.0 0.0 0.15 (1) 6.84 I-F -158 / 122 0.05 (1) H-G -1307 / 0 0.0 0.0 0.14 (1) 7.06 S-M 0 / 1325 0.30 (1) I-G 0 / 1284 0.29 (1) N-M 0 / 0 -28.0 -28.0 0.18 (3) 10.00 M-L 0 / 1286 -28.0 -28.0 0.32 (2) 10.00 L-K 0 / 1030 -28.0 -28.0 0.23 (2) 10.00 K-J 0 / 1030 -28.0 -28.0 0.23 (2) 10.00 J-I 0 / 1257 -28.0 -28.0 0.31 (2) 10.00 I-H 0 / 0 -28.0 -28.0 0.17 (3) 10.00			



DWG NO. TAM 3986-18

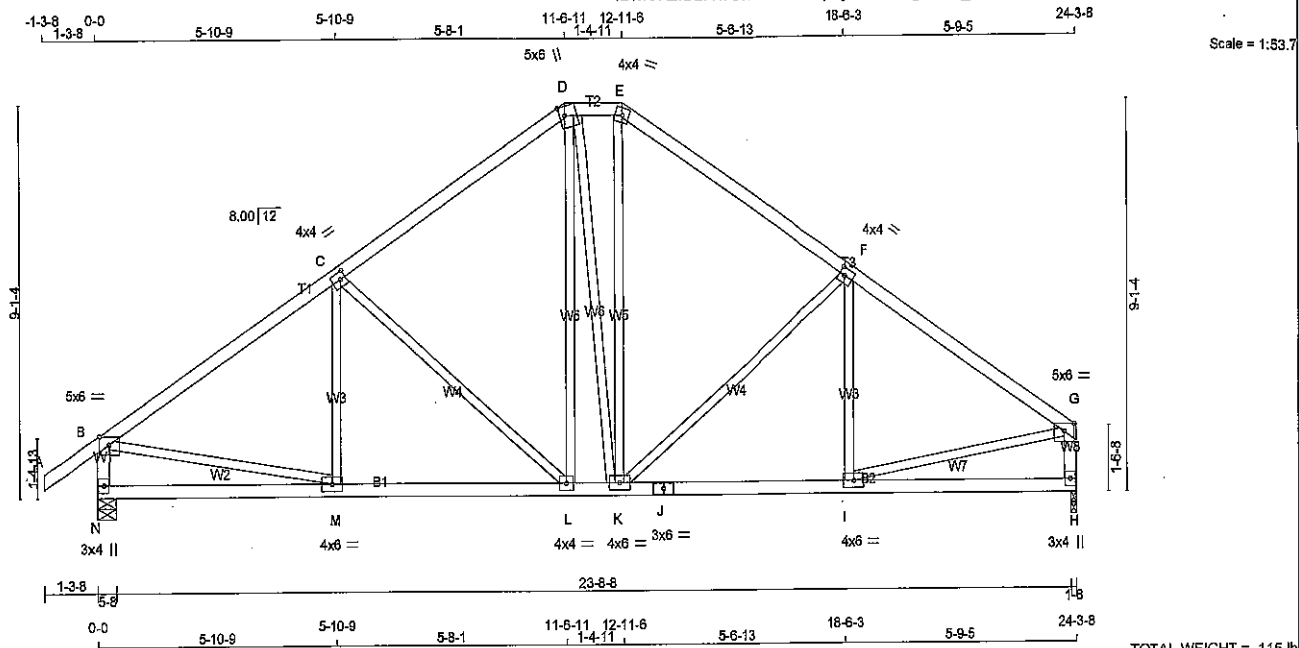
STRUCTURAL

COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
285994	T7A	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 115 lb
[M]F

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
N - B	2x4	DRY	No.2	SPF	
H - G	2x4	DRY	No.2	SPF	
N - J	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	

ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge		
C	TMVW-t	MT20	4.0	4.0	2.00	1.50	
D	TTWW+m	MT20	5.0	6.0	2.50	1.50	
E	TTW-m	MT20	4.0	4.0			
F	TMVW-t	MT20	4.0	4.0	2.00	1.50	
G	TMVW-p	MT20	5.0	6.0	Edge		
H	BMV1+p	MT20	3.0	4.0			
I	BMVW-t	MT20	4.0	6.0			
J	BS-t	MT20	3.0	6.0			
K	BMVW-t	MT20	4.0	6.0			
L	BMVW-t	MT20	4.0	6.0			
M	BMVW-t	MT20	4.0	6.0			
N	BMV1+p	MT20	3.0	4.0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
N	1479	0	1479	0	0	5-8	5-8	5-8	5-8
H	1363	0	1363	0	0	1-8	1-8	1-8	1-8

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1198	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0	0 / 0
H	1119	821 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.86 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		MEMB.		WEBS	
FR-TO	MAX. FACTORED FORCE (LBS)	FR-TO	MAX. FACTORED FORCE (LBS)	FR-TO	MAX. FACTORED FORCE (LBS)
A-B	0 / 32	A-B	-84.3	A-B	-50 / 214
B-C	-1533 / 0	B-C	-84.3	B-C	-481 / 0
C-D	-1163 / 0	C-D	-84.3	C-D	0 / 377
D-E	-945 / 0	D-E	-84.3	D-E	0 / 17
E-F	-1164 / 0	E-F	-84.3	E-F	0 / 390
F-G	-1491 / 0	F-G	-84.3	F-G	-433 / 0
G-H	-1413 / 0	G-H	0.0	G-H	-94 / 185
H-I	-1299 / 0	H-I	0.0	H-I	0 / 1324
I-J	0 / 0	I-J	-28.0	I-J	0 / 1296
J-K	0 / 1302	J-K	-28.0	J-K	0.29 (1)
K-L	0 / 943	K-L	-28.0	K-L	0.05 (3)
L-M	0 / 1268	L-M	-28.0	L-M	0.57 (1)
M-N	0 / 0	M-N	-28.0	M-N	0.08 (1)
N-O	0 / 0	N-O	-28.0	N-O	0.00 (2)
O-P	0 / 0	O-P	-28.0	O-P	0.09 (1)
P-Q	0 / 0	P-Q	-28.0	P-Q	0.51 (1)
Q-R	0 / 0	Q-R	-28.0	Q-R	0.04 (3)
R-S	0 / 0	R-S	-28.0	R-S	0.30 (1)
S-T	0 / 0	S-T	-28.0	S-T	0.29 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 3.0 PSF
BOT CH. LL	= 10.5 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL) = L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CSI: TC=0.42/1.00 (B-C:1), BC=0.40/1.00 (L-M:2), WB=0.57/1.00 (C-L:1), SSI=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR	SECTION
(PSI)	(PLI)
MAX MIN	MAX MIN
MT20	818 354 1687 822 2284 1658

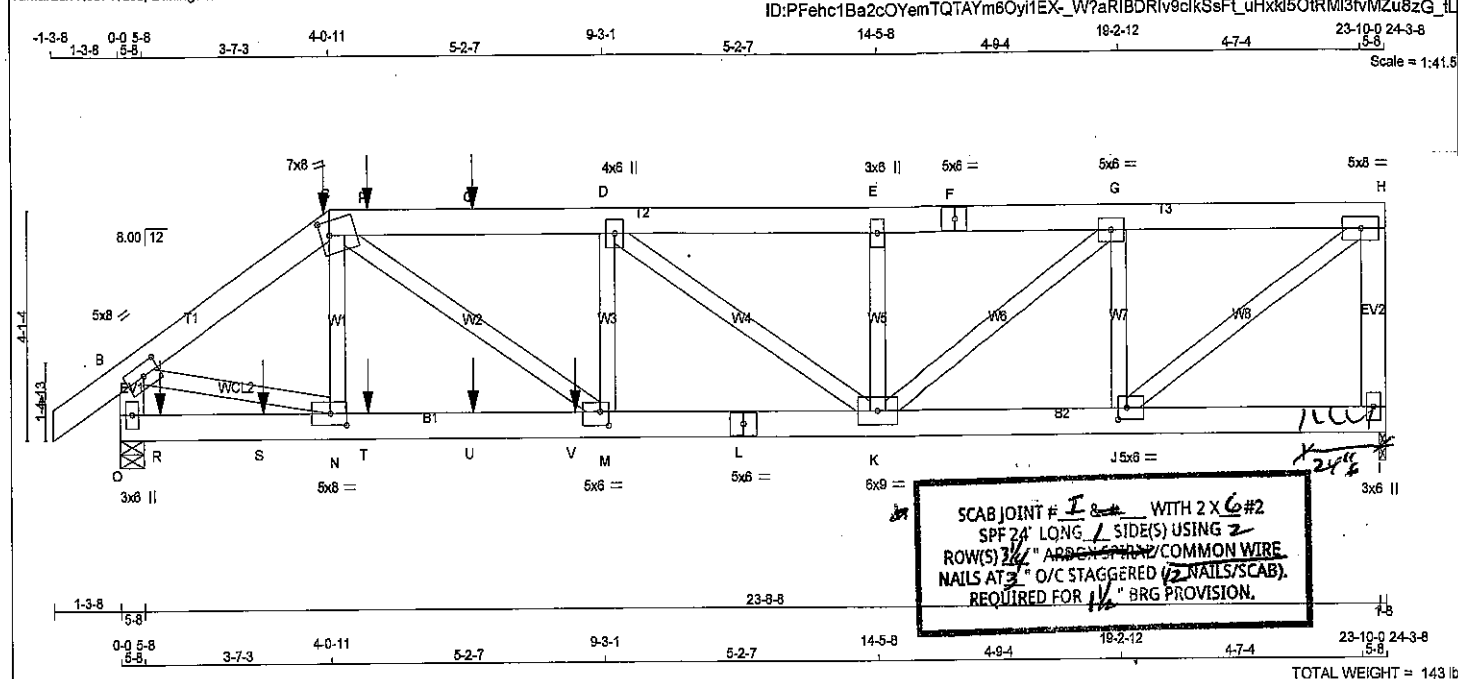
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.33 (M) (INPUT = 1.00)



DWG NO. TAM 3967-18
STRUCTURAL
COMPONENT ONLY



<u>LUMBER</u>				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
I - H	2x6	DRY	No.2	SPF
O - B	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF
EXCEPT				
DRY, SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	BMW-I	MT20	5.0	8.0	2.50 3.75
C	TTWW-m	MT20	7.0	8.0	3.00 2.00
D	TMWW-I	MT20	4.0	8.0	
E	TMW-w	MT20	3.0	6.0	
F	TS-I	MT20	5.0	6.0	
G	TMWW-I	MT20	5.0	6.0	
H	TMW-I	MT20	5.0	8.0	
I	BMW-Ip	MT20	3.0	8.0	
J	BMWW-I	MT20	5.0	6.0	2.50 2.00
K	BMWWW-I	MT20	6.0	9.0	
L	BS-I	MT20	5.0	6.0	
M	BMWW-I	MT20	5.0	6.0	3.00 2.00
N	BMWW-I	MT20	5.0	8.0	2.50 3.75
O	BMW-Ip	MT20	3.0	6.0	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 244.5 lbs FACTORED DOWN AT 4-0-11, AND 113.4 lbs FACTORED DOWN AT 4-9-4, AND 101.4 lbs FACTORED DOWN AT 6-9-4 ON TOP CHORD, AND 75.5 lbs FACTORED DOWN AT 9-4, 69.9 lbs FACTORED DOWN AT 2-9-4, 89.9 lbs FACTORED DOWN AT 4-9-4, AND 69.9 lbs FACTORED DOWN AT 6-9-4, AND 119.0 lbs FACTORED DOWN AT 8-8-6 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REORD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT IN-SX	IN-SX
I	1894	0	1894	0	0	1-8
O	2731	0	2731	0	0	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1548	871 / 0	345 / 0	0 / 0	0 / 0	332 / 0	0 / 0
O	2724	1265 / 0	486 / 0	0 / 0	0 / 0	473 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.73 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY
APPLIED

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

LOADING
TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MAX. FACTORED		FACTORED			MAX. FACTORED				
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRAC	MEMB.	FORCE (LBS)	MAX CSI (LC)		
FR-TO		FROM TO	CSI (LC)	LENGTH	FR-TO				
A-B	0/33	-84.3	-84.3	0.07 (1)	10.00	N-C	-170/231	0.04 (3)	
B-C	-3245/0	-84.3	-84.3	0.19 (1)	4.53	C-M	0/2150	0.38 (1)	
C-P	-4458/0	-84.3	-84.3	0.41 (1)	3.73	M-D	0/237	0.04 (3)	
P-Q	-4458/0	-84.3	-84.3	0.41 (1)	3.73	D-K	-1017/0	0.49 (1)	
Q-D	-4458/0	-84.3	-84.3	0.41 (1)	3.73	K-E	-375/0	0.06 (1)	
D-E	-3829/0	-84.3	-84.3	0.26 (1)	4.27	K-G	0/1673	0.30 (1)	
E-F	-3829/0	-84.3	-84.3	0.22 (1)	4.30	J-G	-1482/0	0.25 (1)	
F-G	-3829/0	-84.3	-84.3	0.22 (1)	4.30	J-H	0/2633	0.50 (1)	
G-H	-2300/0	-84.3	-84.3	0.18 (1)	5.21	B-N	0/2753	0.49 (1)	
I-H	-1833/0	0.0	0.0	0.27 (1)	7.35				
O-B	-2875/0	0.0	0.0	0.19 (1)	6.33				
O-R	0/0	-28.0	-28.0	0.19 (2)	10.00				
R-S	0/0	-28.0	-28.0	0.19 (2)	10.00				
S-N	0/0	-28.0	-28.0	0.19 (2)	10.00				
N-T	0/2686	-28.0	-28.0	0.67 (1)	10.00				
T-U	0/2686	-28.0	-28.0	0.67 (1)	10.00				
U-V	0/2686	-28.0	-28.0	0.67 (1)	10.00				
V-M	0/2686	-28.0	-28.0	0.67 (1)	10.00				
M-L	0/4458	-28.0	-28.0	0.77 (1)	10.00				
L-K	0/4458	-28.0	-28.0	0.77 (1)	10.00				
K-J	0/2300	-28.0	-28.0	0.37 (1)	10.00				
J-I	0/0	-28.0	-28.0	0.08 (3)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX	MAX*	FACE	DIR.	TYPE
P	4-0-11	-23	-25	---	FRONT	VERT	DEAD
C	4-0-11	-222	-222	---	FRONT	VERT	SNOW
O	4-9-4	-113	-113	---	FRONT	VERT	TOTAL
Q	6-8-4	-101	-101	---	FRONT	VERT	TOTAL
R	9-4	-43	-76	---	FRONT	VERT	TOTAL
S	2-6-4	-40	-70	---	FRONT	VERT	TOTAL
T	4-9-4	-40	-70	---	FRONT	VERT	TOTAL
U	6-9-4	-40	-70	---	FRONT	VERT	TOTAL
V	8-8-8	-1157	-1157	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED
BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD =		46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 8.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO
ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= $L/360$ (0.81")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.14")
ALLOWABLE DEFL.(TL)= $L/360$ (0.81")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.22")

CSI: TC=0.41/1.00 (C-D:1), BC=0.77/1.00 (K-M:1),
WB=0.50/1.00 (H-I:1), SSI=0.74/1.00 (M-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

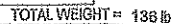
NAIL VALUES					
PLATE	GRIP(DRY)		SHEAR		SECTION
	(PSI)		(PLI)		(PLI)
	MAX	MIN	MAX	MIN	MAX MIN
MT20	818	354	1887	822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.66 (L) (INPUT = 1.00)

DWG NO. TAM 26863-18
 STRUCTURAL
 COMPONENT ONLY



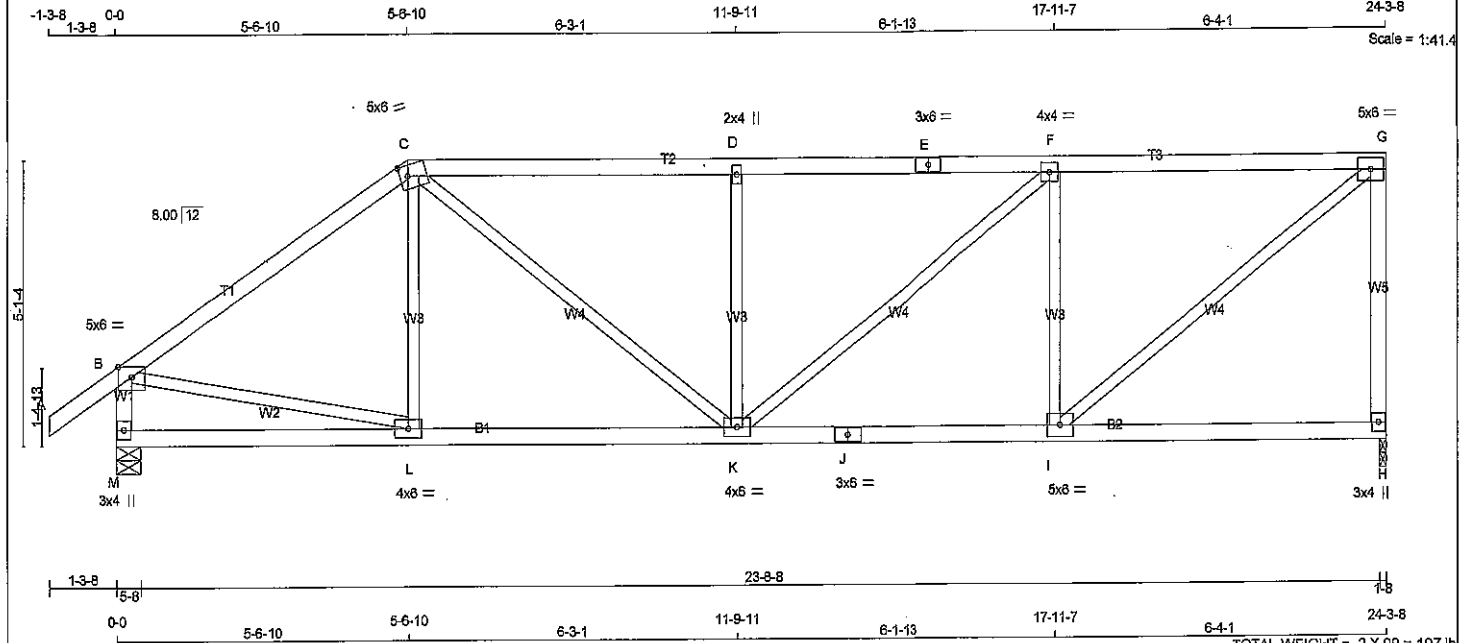
DWG NO. TAM 5089
STRUCTURAL
COMPONENT ONLY

JOB NAME 285994	TRUSS NAME T9	QUANTITY 2	PLY 1	JOB DESC. 43954 TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
H - G	2x4	DRY	No.2
M - B	2x4	DRY	No.2
M - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			SPF
DRY: SEASONED LUMBER.			

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMVW-p	MT20	5.0	6.0 Edge
C	TTWW-m	MT20	5.0	6.0 2.25 1.75
D	TMVW-w	MT20	2.0	4.0
E	TS-t	MT20	3.0	6.0
F	TMVW-t	MT20	4.0	4.0
G	TMVW-t	MT20	5.0	6.0
H	BMV1+p	MT20	3.0	4.0
I	BMVW-t	MT20	5.0	6.0
J	BS-t	MT20	3.0	6.0
K	BMVWW-t	MT20	4.0	6.0
L	BMVW-t	MT20	4.0	6.0
M	BMV1+p	MT20	3.0	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUGHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
H	1363	0	1363	0
M	1479	0	1479	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN.	COMPONENT REACTIONS	WIND	DEAD	SOIL
H	1119	621 / 0	255 / 0	0 / 0	0 / 0	0 / 0
M	1198	692 / 0	255 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.34 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED	FACTORED	MAX.	MEMB.	MAX. FACTORED	FACTORED	MAX.
	FORCE	VERT. LOAD	LC1		FORCE	VERT. LOAD	LC1
	(LBS)	(PLF)	(LC)		(LBS)	(PLF)	(LC)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 32	-84.3	-84.3 0.11 (1)	10.00	L-C	-49 / 204	0.05 (3)
B-C	-1493 / 0	-84.3	-84.3 0.55 (1)	4.89	B-L	0 / 1263	0.28 (1)
C-D	-1797 / 0	-84.3	-84.3 0.55 (1)	4.37	I-G	0 / 1778	0.40 (1)
D-E	-1797 / 0	-84.3	-84.3 0.58 (1)	4.34	C-K	0 / 702	0.18 (1)
E-F	-1797 / 0	-84.3	-84.3 0.58 (1)	4.34	I-F	-877 / 0	0.34 (1)
F-G	-1416 / 0	-84.3	-84.3 0.53 (1)	4.81	K-D	-566 / 0	0.22 (1)
H-G	-1292 / 0	0.0	0.0 0.57 (1)	7.10	K-F	0 / 484	0.11 (1)
M-B	-1417 / 0	0.0	0.0 0.15 (1)	6.85			
M-L	0 / 0	-28.0	-28.0 0.24 (3)	10.00			
L-K	0 / 1240	-28.0	-28.0 0.39 (2)	10.00			
K-J	0 / 1416	-28.0	-28.0 0.44 (2)	10.00			
J-I	0 / 1416	-28.0	-28.0 0.44 (2)	10.00			
I-H	0 / 0	-28.0	-28.0 0.27 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F.
RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED
ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.81")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.09")
ALLOWABLE DEFL.(TL) = $L/360$ (0.81")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.14")

CSI: TC=0.57/1.00 (G-H:1), BC=0.44/1.00 (I-K:2),
WB=0.40/1.00 (G-I:1), SS=0.25/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN
THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1667

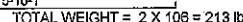
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (K) (INPUT = 0.90)
JSI METAL= 0.44 (J) (INPUT = 1.00)



DRWG NO. TAM 3968-18
STRUCTURAL
COMPONENT ONLY



DRY: SEASONED LUMBER.

PLATES (tablets in inches)

EDITING BEARINGS

BEARINGS			
FACTORED	MAXIMUM FACTORED	INPUT	RECORD

1ST LCASE	MAX./MIN. COMPONENT REACTIONS
1	1
2	2
3	3
4	4
5	5
6	6
7	7
8	8
9	9
10	10
11	11
12	12
13	13
14	14
15	15
16	16
17	17
18	18
19	19
20	20
21	21
22	22
23	23
24	24
25	25
26	26
27	27
28	28
29	29
30	30
31	31
32	32
33	33
34	34
35	35
36	36
37	37
38	38
39	39
40	40
41	41
42	42
43	43
44	44
45	45
46	46
47	47
48	48
49	49
50	50
51	51
52	52
53	53
54	54
55	55
56	56
57	57
58	58
59	59
60	60
61	61
62	62
63	63
64	64
65	65
66	66
67	67
68	68
69	69
70	70
71	71
72	72
73	73
74	74
75	75
76	76
77	77
78	78
79	79
80	80
81	81
82	82
83	83
84	84
85	85
86	86
87	87
88	88
89	89
90	90
91	91
92	92
93	93
94	94
95	95
96	96
97	97
98	98
99	99
100	100

		MAX-MIN. COMB. SENSIT. REACTIONS					
.IT	1ST CASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.86 FT.

LOADING

TOTAL LOAD CASES: (4)

CHORDS WEBS

WEBS

MAX. FACTORED

N-M	0/1196	-28.0	-28.0	0.45 (2)	10.00
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SPECIFIED LOADS

TOP CH. LL = 25.6 PSF

TOTAL LOAD	46.1	1.61
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SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A

THIS TRUSS IS DESIGNED FOR RESIDENTIAL

THIS DESIGN COMPLIES WITH:

/55% OF 313 PSE GSI PLUS 84 PSE

ALLOWABLE DEF. (LL) = 1/360 (0.81")

CS: TC=0.94/1.00 (H-F:1) BC=0.47/1.00 (I-M:2)

DO1 LUMBER=1.00 NAIL=1.00 L S BEND=1.10

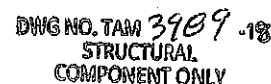
COMPANION LIVE LOAD FACTOR = 0.50

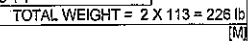
TRUSS PLATE MANUFACTURER IS NOT

NAIL VALUES

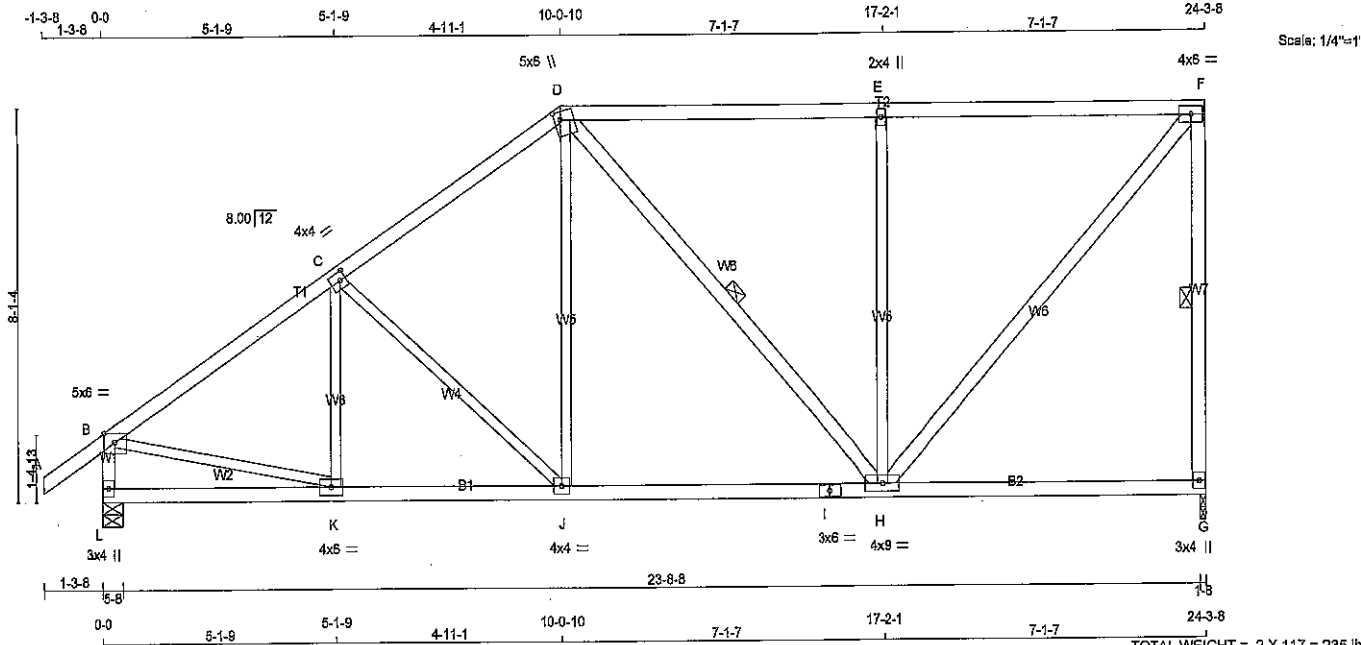
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg

 $ISI_{GRIP} = 0.80 (D) / (INPUT = 0.90)$ 



DWG NO. TAM 3990-18
STRUCTURAL
COMPONENT ONLY



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
D - F	2x4	DRY No.2	SPF
G - F	2x4	DRY No.2	SPF
L - B	2x4	DRY No.2	SPF
L - I	2x4	DRY No.2	SPF
I - G	2x4	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF
D - H	2x4	DRY No.2	SPF
H - F	2x4	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TTVW+rn	MT20	5.0	6.0	2.00	1.50
E	TMVW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMVWV-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMVW-t	MT20	4.0	4.0		
K	BMVW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
G	1363	0	1363	0	1-8	1-8
L	1479	0	1479	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
G	1119	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
L	1198	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.81 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	CHORDS		MAX. MEMB. UNBRACED LENGTH FR-TO	WEBS	
		VERT. LOAD (PLF)	LC1 MAX CSI (LC)		MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM	TO			
A-B	0 / 32	-84.3	-84.3 0.11 (1)	10.00	K-C	-123 / 133 0.04 (1)
B-C	-1529 / 0	-84.3	-84.3 0.43 (1)	4.83	C-J	-347 / 0 0.29 (1)
C-D	-1275 / 0	-84.3	-84.3 0.40 (1)	5.20	J-D	0 / 436 0.10 (2)
D-E	-987 / 0	-84.3	-84.3 0.80 (1)	4.81	D-H	-110 / 0 0.08 (1)
E-F	-987 / 0	-84.3	-84.3 0.81 (1)	4.81	H-E	-745 / 0 0.94 (1)
G-F	-1284 / 0	0.0	0.0 0.38 (1)	5.89	H-F	0 / 1432 0.23 (1)
L-B	-1418 / 0	0.0	0.0 0.15 (1)	6.85	B-K	0 / 1323 0.30 (1)
L-K	0 / 0	-28.0	-28.0 0.16 (3)	10.00		
K-J	0 / 1295	-28.0	-28.0 0.33 (2)	10.00		
J-I	0 / 1041	-28.0	-28.0 0.49 (2)	10.00		
I-H	0 / 1041	-28.0	-28.0 0.49 (2)	10.00		
H-G	0 / 0	-28.0	-28.0 0.37 (3)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	PSF
	25.6	PSF
	DL	3.0 PSF
BOT CH.	LL	10.5 PSF
	DL	7.0 PSF
TOTAL LOAD		46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/999 (0.10")
ALLOWABLE DEFL.(TL) = L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/999 (0.16")

CSI: TC=0.81/1.00 (E-F:1), BC=0.49/1.00 (H-J:2), WB=0.94/1.00 (E-H:1), SSI=0.29/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1687 822 2284 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL= 0.36 (I) (INPUT = 1.00)



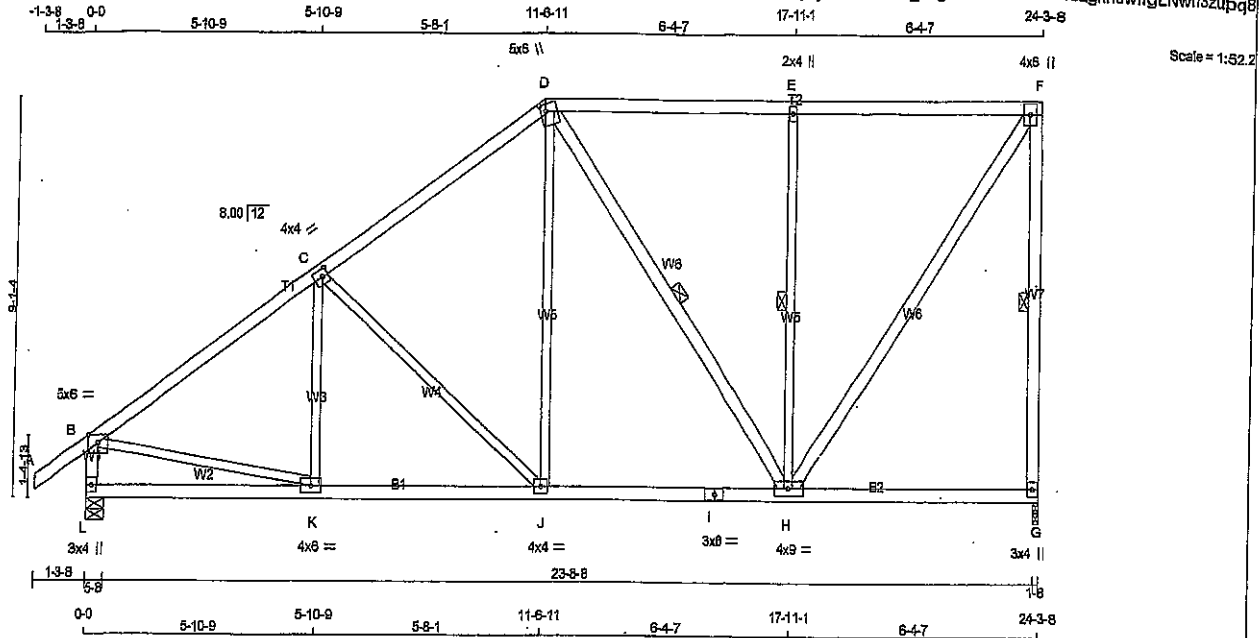
DRWG NO. TAM 3991-18
STRUCTURAL
COMPONENT ONLY

JOB NAME 286039	TRUSS NAME T13	QUANTITY 5	PLY 1	JOB DESC. 43954 TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:w3K2IB2K4FGk9ID07SkJphyfMC-9n5zUJ_wsgWKT4XUsZLEIM6zgndwifgLNwm3zupq8



TOTAL WEIGHT = 5 X 123 = 614 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
G - F	2x4	DRY	No.2
L - B	2x4	DRY	No.2
L - I	2x4	DRY	No.2
I - G	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
D - H	2x4	DRY	No.2
H - F	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
B TMVW-p	MT20	5.0	8.0	Edge
C TMVW-t	MT20	4.0	4.0	2.00 1.50
D TTWW+m	MT20	5.0	6.0	2.00 1.50
E TMVW-w	MT20	2.0	4.0	
F TMVW+p	MT20	4.0	6.0	
G BMV1+p	MT20	3.0	4.0	
H BMVWW-t	MT20	4.0	9.0	
I BS-t	MT20	3.0	6.0	
J BMVW-t	MT20	4.0	4.0	
K BMVW-t	MT20	4.0	6.0	
L BMV1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
G	1393	0	1393	0	0	1-8	1-8	1-8	1-8
L	1479	0	1479	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
G	1119	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
L	1198	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.01 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 32	K-C	-58 / 203
B-C	-1531 / 0	C-J	-474 / 0
C-D	-1167 / 0	J-D	0 / 490
D-E	-790 / 0	D-H	-270 / 0
E-F	-790 / 0	H-E	-688 / 0
G-F	-1292 / 0	H-F	0 / 1344
L-B	-1412 / 0	B-K	0 / 1322
L-K	0 / 0		
K-J	0 / 1300		
J-I	0 / 948		
I-H	0 / 948		
H-G	0 / 0		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 3.0 PSF
BOT CH. LL = 10.8 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CSI: TC=0.62/1.00 (E-F:1), BC=0.40/1.00 (H-J:2), WB=0.58/1.00 (C-J:1), SSI=0.28/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1887 622 2284 1858

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.32 (K) (INPUT = 1.00)



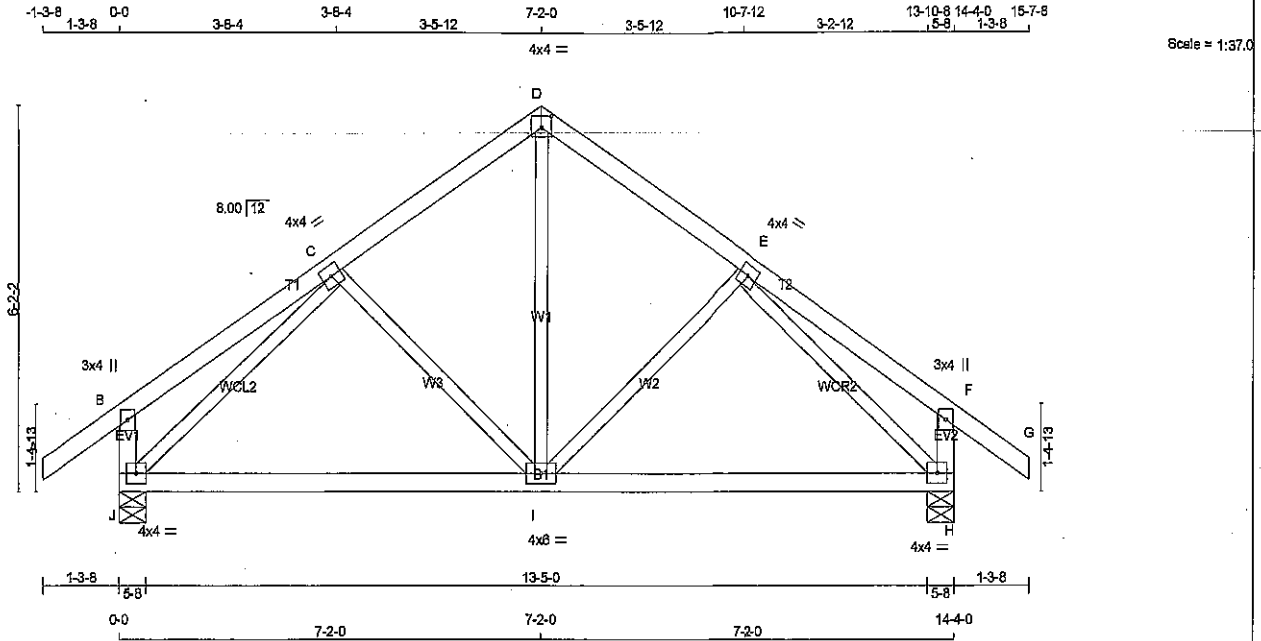
DRWG NO. TAM 3993 -18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285746	T14	6	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:Uar6HFwTUsfudnkBu50lkHyEuR-PfXXBPPZLPDzTmzausindqMe_R_FIQAMBgg4A8zP94q



TOTAL WEIGHT = 6 X 62 = 371 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
J - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
C	TMWW-t	MT20	4.0	4.0	
D	TTW-p	MT20	4.0	4.0	2.25 2.00
E	TMWW-t	MT20	4.0	4.0	
F	TMV+p	MT20	3.0	4.0	
H	BMVW1-t	MT20	4.0	4.0	
I	BMVW1-t	MT20	4.0	6.0	
J	BMVW1-t	MT20	4.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
J	920	0	920	0	0	5-8	5-8	5-8	5-8
H	920	0	920	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	739	437 / 0	151 / 0	0 / 0	0 / 0	152 / 0	0 / 0
H	739	437 / 0	151 / 0	0 / 0	0 / 0	152 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX	MEMB.	FORCE (LBS)
FR-TO		FROM	TO	FR-TO	
A-B	0 / 32	-84.3	-84.3 0.11 (1)	I-D	0 / 460
B-C	0 / 19	-84.3	-84.3 0.17 (1)	I-E	-152 / 44
C-D	-845 / 0	-84.3	-84.3 0.13 (1)	C-I	-152 / 44
D-E	-845 / 0	-84.3	-84.3 0.13 (1)	J-C	-879 / 0
E-F	0 / 19	-84.3	-84.3 0.17 (1)	E-H	-679 / 0
F-G	0 / 32	-84.3	-84.3 0.11 (1)		
J-B	-232 / 0	0.0	0.0 0.02 (1)		
H-F	-232 / 0	0.0	0.0 0.02 (1)		
J-I	0 / 831	-28.0	-28.0 0.49 (2)		
I-H	0 / 631	-28.0	-28.0 0.49 (2)		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 3.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.48")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.48")
CALCULATED VERT. DEFL.(TL) = L/999 (0.13")

CSI: TC=0.17/1.00 (E-F:1), BC=0.49/1.00 (H-I:2),
WB=0.35/1.00 (E-H:1), SS=0.17/1.00 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (D) (INPUT = 0.90)
JSI METAL= 0.31 (C) (INPUT = 1.00)



DWG NO. TAM 2148-18
STRUCTURAL
COMPONENT ONLY

Tamarack Roof Truss, Burlington

Version 8.210 S May 18 2018 MTEK Industries, Inc. Tue



TOTAL WEIGHT = 5 X 22 = 110 #

DRY: SEASONED LUMBER.

PLATES (table is in inches)

BEARINGS

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT H

UNFACTORED REACTIONS

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S).

PLACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATEROALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

J-1	0 / 631	-28.0	-28.0	0.49 (2)	10.00
I-H	0 / 631	-28.0	-28.0	0.49 (2)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:

SPACING 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 0.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL (LL) = $L/360$ (0.48")
 CALCULATED VERT. DEFL (LL) = $L/999$ (0.08")
 ALLOWABLE DEFL (TL) = $L/360$ (0.48")
 CALCULATED VERT. DEFL (TL) = $L/809$ (0.13")

CSI: TC=0.17/1.00 (E-F:1), BC=0.49/1.00 (H-I:2),
WB=0.35/1.00 (E-H:1), SS=0.17/1.00 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

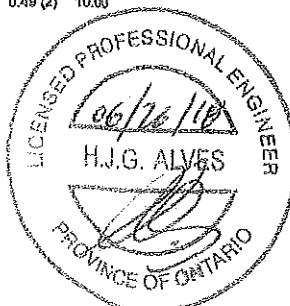
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg

JSI GRIP = 0.77 (D) (INPUT = 0.90)

JSI METAL = 0.31 (C) (INPUT = 1.00)

DWG NO. TAM 1800643
STRUCTURAL
COMPONENT ONLY

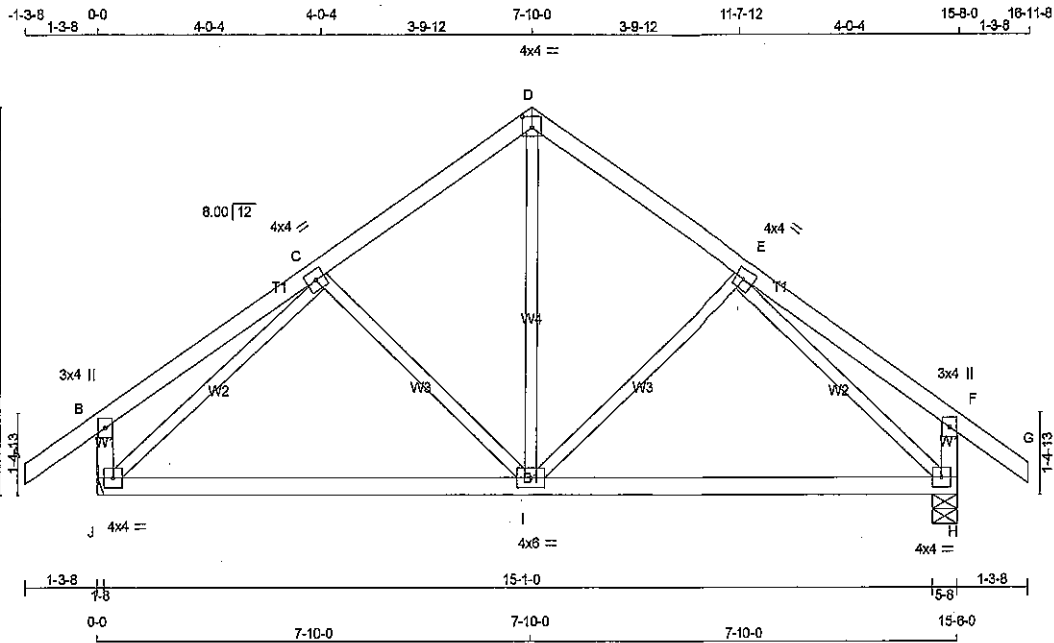


JOB NAME 285861	TRUSS NAME T15	QUANTITY 1	PLY 1	JOB DESC. 43954 TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:w3K2IB2K4fGk9tDO7SkJphylfMC-ueeqmRT0PAR97mBavoxoqfTrVcBxAPxm3xpOo0yg2th



Scale = 1:39.6

TOTAL WEIGHT = 67 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - G	2x4	DRY	No.2	SPF	
J - B	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
J	995	0	995	0
H	995	0	995	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED	SNOW	LIVE			
J	800	471 / 0	185 / 0	0 / 0	165 / 0	0 / 0
H	800	471 / 0	185 / 0	0 / 0	165 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)	
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 32	-84.3	-84.3	0.11 (1)	I-D	0 / 516	0.12 (1)
B-C	0 / 21	-84.3	-84.3	0.20 (1)	I-E	-179 / 43	0.09 (1)
C-D	-717 / 0	-84.3	-84.3	0.18 (1)	C-I	-179 / 43	0.09 (1)
D-E	-717 / 0	-84.3	-84.3	0.16 (1)	J-C	-975 / 0	0.46 (1)
E-F	0 / 21	-84.3	-84.3	0.20 (1)	E-H	-975 / 0	0.46 (1)
F-G	0 / 32	-84.3	-84.3	0.11 (1)			
J-B	-243 / 0	0.0	0.0	0.02 (1)			
H-F	-243 / 0	0.0	0.0	0.02 (1)			
J-I	0 / 709	-28.0	-28.0	0.58 (2)			
I-H	0 / 709	-28.0	-28.0	0.58 (2)			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.52")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.11")
ALLOWABLE DEFL.(TL) = $L/360$ (0.52")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.18")

CSI: TC=0.20 (B-C:1), BC=0.58 (H-I:2), WB=0.48 (C-J:1), SSI=0.19 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 622 2284 1655

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (D) (INPUT = 0.90)
JSI METAL= 0.35 (C) (INPUT = 1.00)

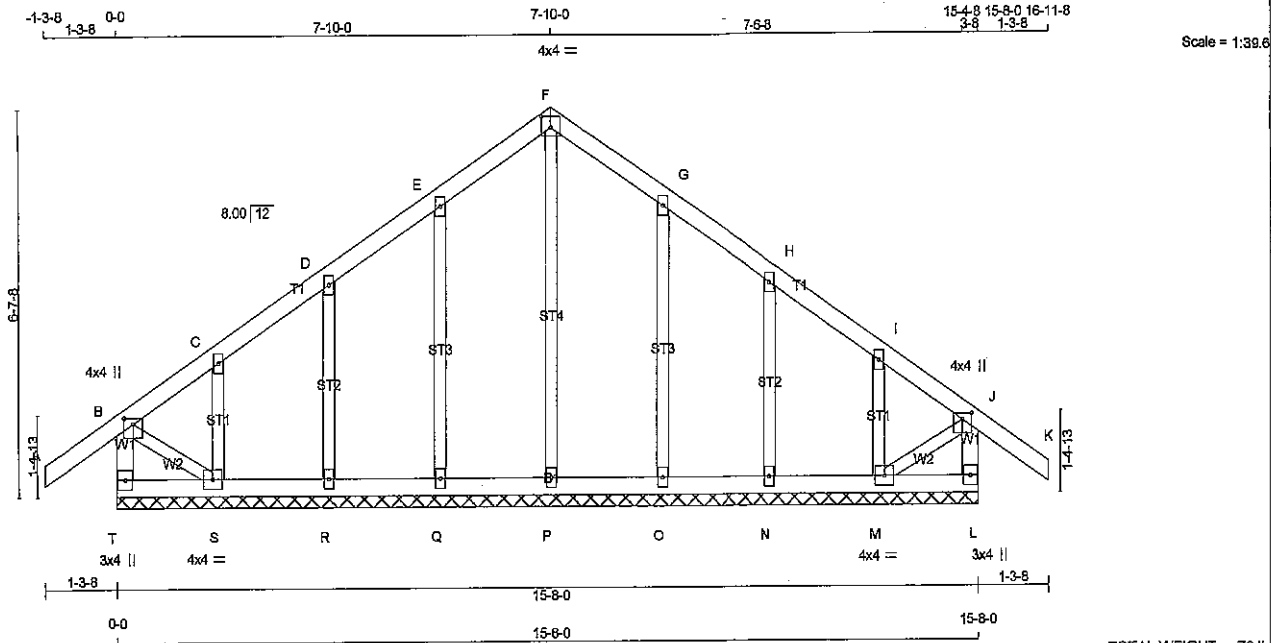


DWG NO. TAM 45080 17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
285861	G15	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1:38.6

TOTAL WEIGHT = 70 lb

LUMBER	DESCR.
N. L. G. A. RULES	
CHORDS	SIZE
T - B	2x4 DRY No.2 SPF
A - F	2x4 DRY No.2 SPF
F - K	2x4 DRY No.2 SPF
L - J	2x4 DRY No.2 SPF
T - L	2x4 DRY No.2 SPF
ALL WEBS	2x3 DRY No.2 SPF
ALL GABLE WEBS	2x3 DRY No.2 SPF
DRY: SEASONED LUMBER.	
GABLE STUDS SPACED AT	2-0-0 OC.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 3.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11 (J-K-1), BC=0.03 (R-S-2), WB=0.09 (F-P-1), SSI=0.08 (J-K-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	822	2284	1658

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.18 (B) (INPUT = 0.90)

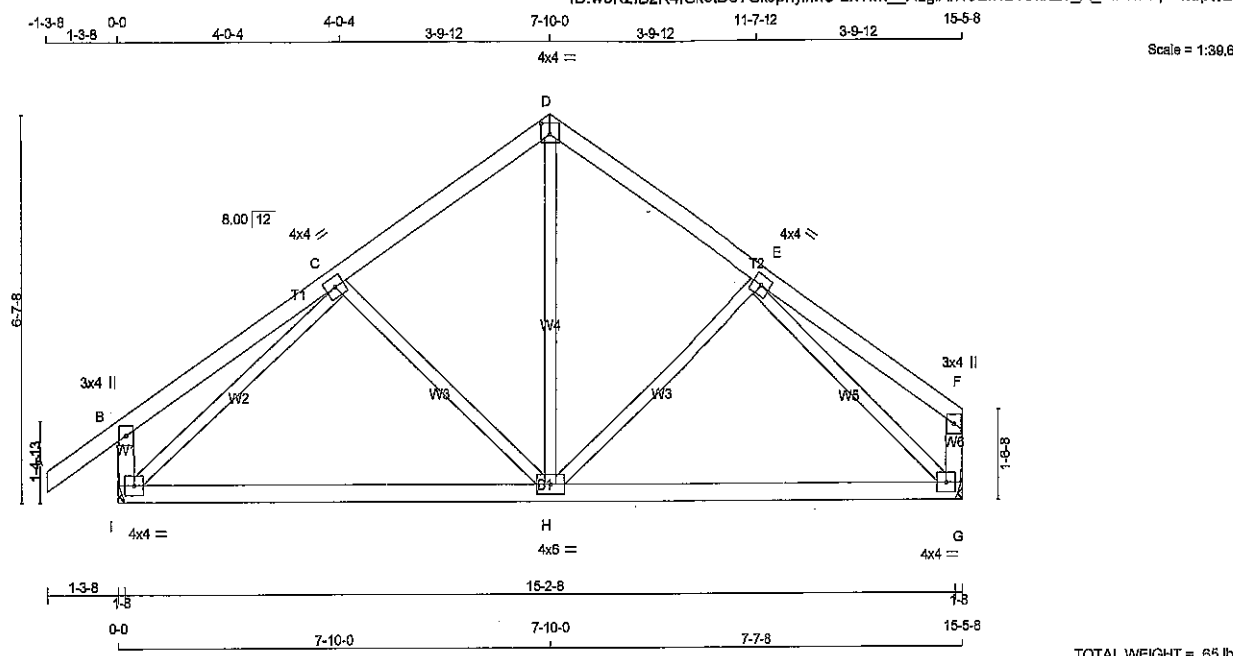
JSI METAL= 0.05 (G) (INPUT = 1.00)

<u>PLATES (table is in inches)</u>					
JT	TYPE	PLATES	W	LEN	Y X
B	TMW+W+p	MT20	4.0	4.0	1.25 2.00
C, D, E, G, H, I					
C	TMW+W	MT20	2.0	4.0	
F	TTW-p	MT20	4.0	4.0	2.25 2.00
J	TMW+W+p	MT20	4.0	4.0	1.25 2.00
L	BMV1+p	MT20	3.0	4.0	
M	BMW1-t	MT20	4.0	4.0	
N, O, P, Q, R					
N	BMV1+W	MT20	2.0	4.0	
S	BMW1-t	MT20	4.0	4.0	
T	BMV1+p	MT20	3.0	4.0	

CHORDS	WEBS
MEMB.	MEMB.
MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
VERT. LOAD LC1 (PLF)	MAX. UNBRACED LENGTH (LC)
FROM	TO
FR-TO	FR-TO
T-B	-253 / 0
A-B	0 / 32
B-C	-46 / 0
C-D	-15 / 0
D-E	-14 / 0
E-F	-21 / 0
F-G	-21 / 0
G-H	-14 / 0
H-I	-15 / 0
I-J	-48 / 0
J-K	0 / 32
L-J	-253 / 0
T-S	0 / 0
S-R	0 / 16
R-Q	0 / 12
Q-P	0 / 9
P-O	0 / 9
O-N	0 / 12
N-M	0 / 16
M-L	0 / 0

DWG NO. TAM 45735-17
STRUCTURAL
COMPONENT ONLY





TOTAL WEIGHT = 65 lb (M/F)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	4.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMVW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
I	983	0	983	0	0	0
G	868	0	868	0	0	0

MIN. SEAT SIZE: 1-8
HANGER BY OTHERS

UNFACTORED REACTIONS							
JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	791	468 / 0	162 / 0	0 / 0	0 / 0	183 / 0	0 / 0
G	712	395 / 0	162 / 0	0 / 0	0 / 0	155 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD LC1 (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD LC1 (LC)
FR-TO		FROM TO			FR-TO		
A-B	0/32	-84.3	-84.3	0.11 (1)	10.00	H-D	0/494 0.11 (1)
B-C	0/21	-84.3	-84.3	0.20 (1)	10.00	H-E	-148/53 0.07 (1)
C-D	-698/0	-84.3	-84.3	0.16 (1)	6.25	C-H	-182/41 0.09 (1)
D-E	-697/0	-84.3	-84.3	0.15 (1)	6.25	I-C	-957/0 0.45 (1)
E-F	0/21	-84.3	-84.3	0.19 (1)	10.00	E-G	-948/0 0.42 (1)
I-B	-242/0	0.0	0.0	0.02 (1)	7.81		
G-F	-118/0	0.0	0.0	0.01 (1)	7.81		
I-H	0/686	-28.0	-28.0	0.56 (2)	10.00		
H-G	0/672	-28.0	-28.0	0.56 (2)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.52")
CALCULATED VERT. DEFL. (LL) = L/969 (0.11")
ALLOWABLE DEFL. (TL) = L/360 (0.52")
CALCULATED VERT. DEFL. (TL) = L/981 (0.19")

CSI: TC=0.20/1.00 (B-C:1), BC=0.56/1.00 (H-I:2), WB=0.45/1.00 (C-I:1), SS=0.19/1.00 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

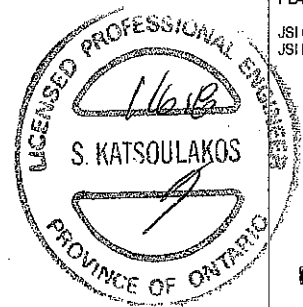
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (D) (INPUT = 0.90)
JSI METAL= 0.34 (C) (INPUT = 1.00)

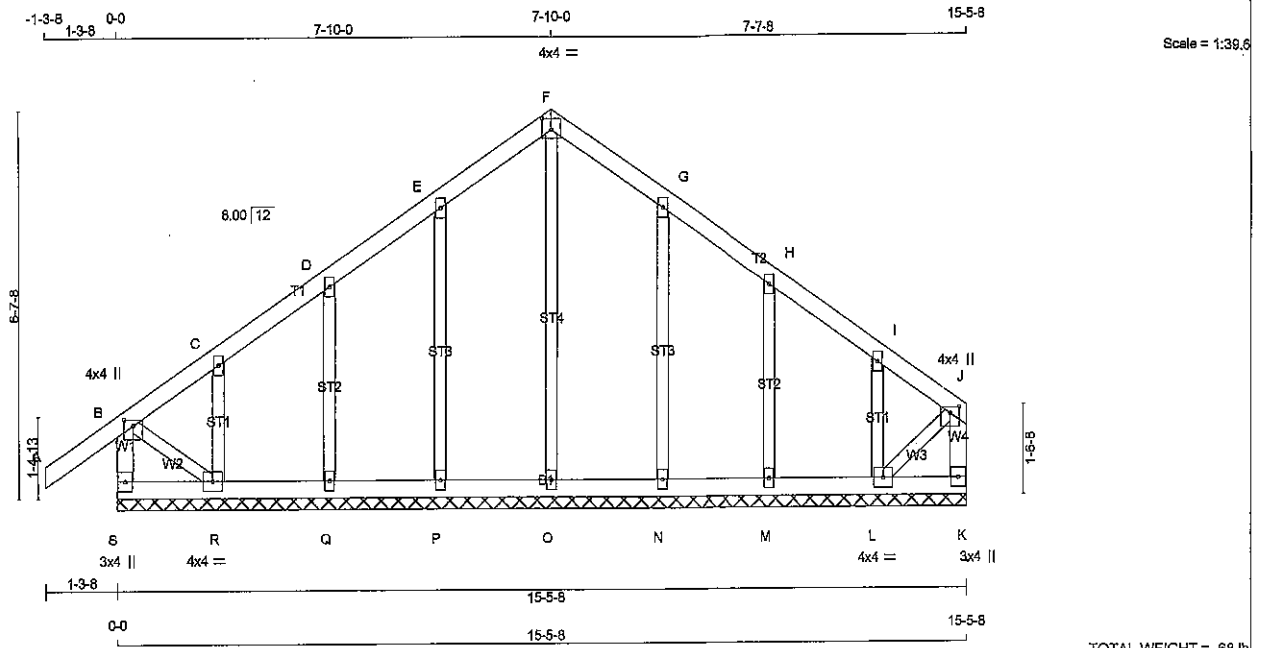


DRWG NO. TAM 4010-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
285861	G15A	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.030.S.Oct.5.2016 MITek Industries, Inc. Thu Sep 07 11:08:54 2017. Page 1.
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TOTAL WEIGHT = 68 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR	
S - B	2x4	DRY	No.2	SPF
A - F	2x4	DRY	No.2	SPF
F - J	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
S - K	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

GABLE STUDS SPACED AT 24-0 OC.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	4.0	1.25	2.00
C, D, E, G, H, I					
C TMVW+w	MT20	2.0	4.0		
F TTW-p	MT20	4.0	4.0	2.25	2.00
J TMVW+p	MT20	4.0	4.0	1.25	2.00
K BMV1+p	MT20	3.0	4.0		
L BMVW1-t	MT20	4.0	4.0		
M, N, O, P, Q					
M BMV1+w	MT20	2.0	4.0		
R BMVW1-t	MT20	4.0	4.0		
S BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)	UNBRACED LENGTH
FR-TO				FR-TO			
S-B	-258 / 0	0.0	0.0 0.03 (1)	7.81	O-F	-124 / 0	0.09 (1)
A-B	0 / 32	-84.3	-84.3 0.11 (1)	10.00	P-E	-189 / 0	0.08 (1)
B-C	-80 / 0	-84.3	-84.3 0.11 (1)	6.25	Q-D	-172 / 0	0.04 (1)
C-D	-18 / 0	-84.3	-84.3 0.04 (1)	6.25	R-C	-108 / 0	0.02 (1)
D-E	-18 / 0	-84.3	-84.3 0.05 (1)	6.25	N-G	-191 / 0	0.08 (1)
E-F	-25 / 0	-84.3	-84.3 0.05 (1)	6.25	M-H	-182 / 0	0.04 (1)
F-G	-25 / 0	-84.3	-84.3 0.05 (1)	6.25	L-I	-162 / 0	0.03 (1)
G-H	-17 / 0	-84.3	-84.3 0.05 (1)	6.25	B-R	0 / 28	0.01 (1)
H-I	-24 / 0	-84.3	-84.3 0.04 (1)	6.25	L-J	0 / 32	0.01 (1)
I-J	-21 / 0	-84.3	-84.3 0.04 (1)	6.25			
K-J	-84 / 0	0.0	0.0 0.01 (1)	7.81			
S-R	0 / 0	-28.0	-28.0 0.03 (2)	10.00			
R-Q	0 / 20	-28.0	-28.0 0.03 (2)	10.00			
Q-P	0 / 18	-28.0	-28.0 0.02 (2)	10.00			
P-O	0 / 13	-28.0	-28.0 0.02 (2)	10.00			
O-N	0 / 13	-28.0	-28.0 0.02 (2)	10.00			
N-M	0 / 16	-28.0	-28.0 0.02 (2)	10.00			
M-L	0 / 20	-28.0	-28.0 0.02 (2)	10.00			
L-K	0 / 0	-28.0	-28.0 0.02 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	25.6	PSF
DL	=	3.0	PSF
BOT CH. LL	=	10.5	PSF
DL	=	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11 (A-B:1), BC=0.03 (Q-R:2), WB=0.09 (F-O:1), SS=0.08 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1567
	822	2284	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (B) (INPUT = 0.90)

JSI METAL= 0.05 (G) (INPUT = 1.00)



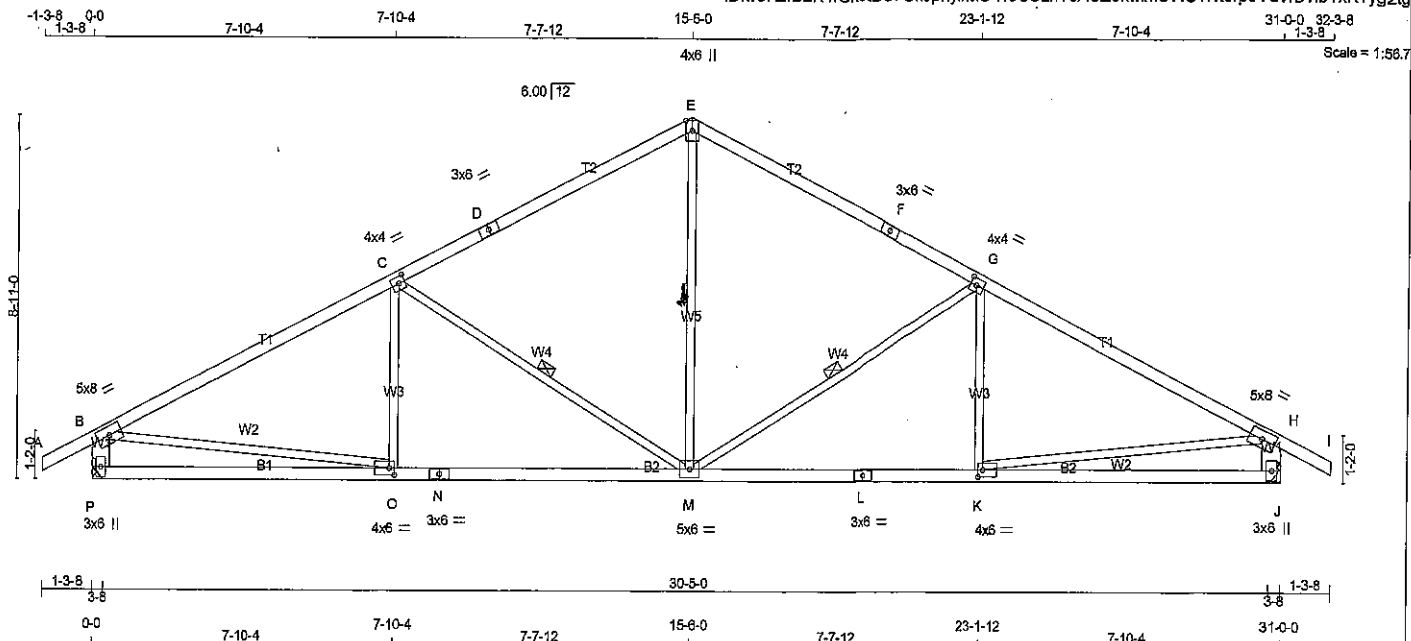
DRWG NO. TAN 45736-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
285861	T24	10	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 10 X 122 = 1216 lb

LUMBER	N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS				
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
P - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
DRY: SEASONED LUMBER.				

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ
JT	1854	0	1854	0
P	1854	0	1854	0
J	1854	0	1854	0

UNFACTORED REACTIONS

	1ST LCASE	MAX/MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	1508	863 / 0	328 / 0	0 / 0	0 / 0	316 / 0	0 / 0
J	1508	863 / 0	328 / 0	0 / 0	0 / 0	316 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.27 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-M, C-M, E-N

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	FACTORED VERT. LOAD (PLF)	MAX. CS (LC)	UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)
FR-TO							FR-TO			
A-B	0 / 28	-84.3	-84.3	0.11 (1)	10.00		M-E	0 / 1096	0.25 (1)	
B-C	-2529 / 0	-84.3	-84.3	0.94 (1)	3.27		M-G	-840 / 0	0.48 (1)	
C-D	-1803 / 0	-84.3	-84.3	0.78 (1)	3.98		K-G	-9 / 307	0.07 (3)	
D-E	-1803 / 0	-84.3	-84.3	0.78 (1)	3.98		C-M	-840 / 0	0.48 (1)	
E-F	-1803 / 0	-84.3	-84.3	0.78 (1)	3.98		O-C	-9 / 307	0.07 (3)	
F-G	-1803 / 0	-84.3	-84.3	0.78 (1)	3.98		B-O	0 / 2308	0.52 (1)	
G-H	-2529 / 0	-84.3	-84.3	0.94 (1)	3.27		K-H	0 / 2308	0.52 (1)	
H-I	0 / 28	-84.3	-84.3	0.11 (1)	10.00					
P-B	-1784 / 0	0.0	0.0	0.11 (1)	7.48					
J-H	-1784 / 0	0.0	0.0	0.11 (1)	7.48					
P-O	0 / 0	-28.0	-28.0	0.42 (3)	10.00					
O-N	0 / 2293	-28.0	-28.0	0.68 (2)	10.00					
N-M	0 / 2293	-28.0	-28.0	0.68 (2)	10.00					
M-L	0 / 2293	-28.0	-28.0	0.68 (2)	10.00					
L-K	0 / 2293	-28.0	-28.0	0.68 (2)	10.00					
K-J	0 / 0	-28.0	-28.0	0.42 (3)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
DL = 3.0 PSF	
BOT CH.	LL = 10.5 PSF
DL = 7.0 PSF	
TOTAL LOAD	= 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/999 (0.18")
ALLOWABLE DEFL.(TL) = L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/999 (0.30")

CSI: TC=0.94 (G-H:1), BC=0.68 (K-M:2), WB=0.52 (H-K:1), SSI=0.29 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

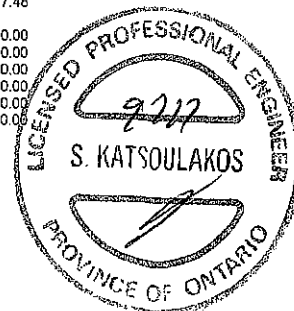
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	618	354	1687
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (K) (INPUT = 0.90)
JSI METAL= 0.69 (L) (INPUT = 1.00)



DWG NO. TAM 15DB2-17
STRUCTURAL
COMPONENT ONLY

JOB NAME 286043	TRUSS NAME T30	QUANTITY 1	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
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FACTORED CONCENTRATED LOADS (LBS)						
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR. TYPE
T	5-3-12	-40	-70	—	BACK	VERT TOTAL
U	7-3-12	-40	-70	—	BACK	VERT TOTAL
V	9-3-12	-40	-70	—	BACK	VERT TOTAL
W	11-3-12	-40	-70	—	BACK	VERT TOTAL
X	13-3-12	-40	-70	—	BACK	VERT TOTAL
Y	15-3-12	-47	-83	—	BACK	VERT TOTAL

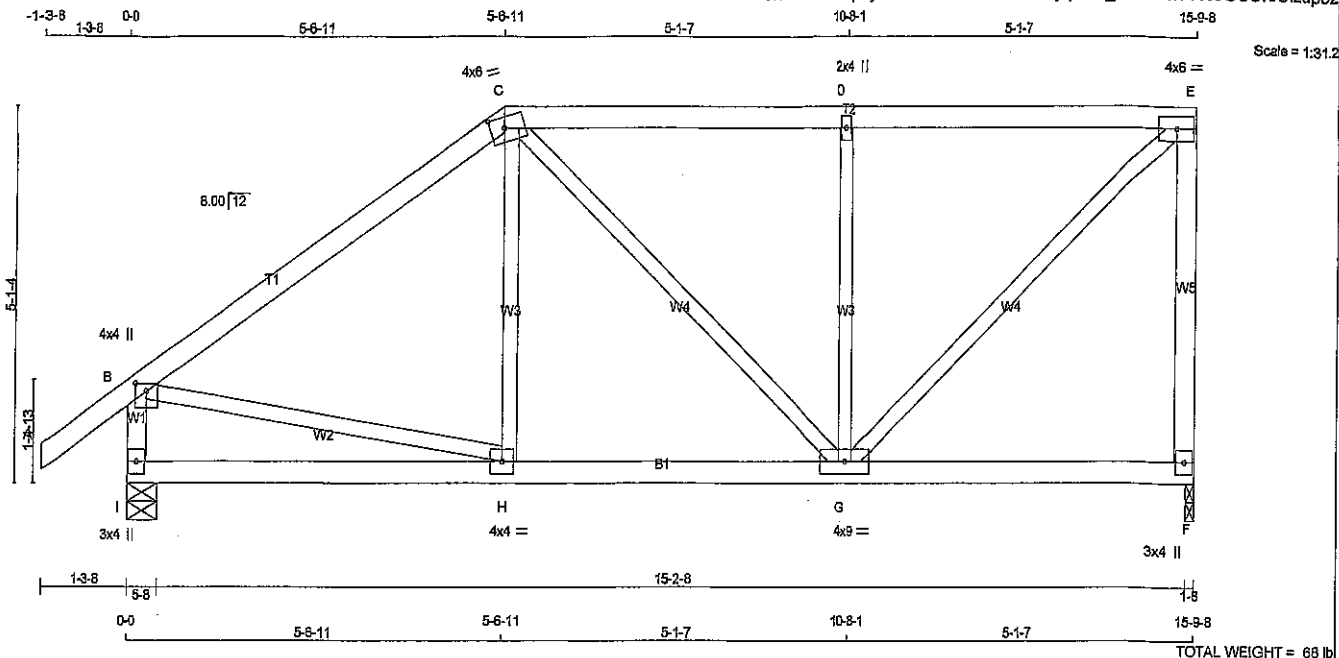


DWG NO. TAM 3995 -18
STRUCTURAL
COMPONENT ONLY

JOB NAME 286043	TRUSS NAME T31	QUANTITY 1	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	SPF	
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
F - E	2x4	DRY	No.2	SPF	
I - B	2x4	DRY	No.2	SPF	
I - F	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW+p	MT20	4.0	4.0	1.25	2.00
C	TTWW-m	MT20	4.0	6.0	1.75	2.50
D	TMW+w	MT20	2.0	4.0		
E	TMW+t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWW+t	MT20	4.0	9.0		
H	BMWW-t	MT20	4.0	4.0		
I	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED		MAXIMUM FACTORED		INPUT		REQD	
GROSS REACTION		GROSS REACTION		BRG		BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	886	0	886	0	0	1-8	1-8
I	1002	0	1002	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	728	404 / 0	166 / 0	0 / 0	0 / 0	158 / 0	0 / 0
I	806	474 / 0	166 / 0	0 / 0	0 / 0	166 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 (LC)	MEMB.	FORCE (LBS)	LC1 (LC)	MEMB.
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 32	-84.3	-84.3 0.11 (1)	10.00	H-C	0 / 222	0.05 (3)
B-C	-821 / 0	-84.3	-84.3 0.49 (1)	5.98	C-G	-23 / 33	0.02 (3)
C-D	-707 / 0	-84.3	-84.3 0.38 (1)	6.25	G-D	-532 / 0	0.20 (1)
D-E	-707 / 0	-84.3	-84.3 0.38 (1)	6.25	G-E	0 / 967	0.22 (1)
F-E	-826 / 0	0.0	0.0 0.38 (1)	7.81	B-H	0 / 695	0.16 (1)
I-B	-837 / 0	0.0	0.0 0.10 (1)	7.81			
I-H	0 / 0	-28.0	-28.0 0.19 (3)	10.00			
H-G	0 / 683	-28.0	-28.0 0.28 (2)	10.00			
G-F	0 / 0	-28.0	-28.0 0.16 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.8	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN./G/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.53")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.05")
ALLOWABLE DEFL.(TL) = $L/360$ (0.53")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.07")

CSI: TC=0.49/1.00 (B-C:1), BC=0.28/1.00 (G-H:2), WB=0.22/1.00 (E-G:1), SI=0.21/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	618	354	1687
	822	2284	1656

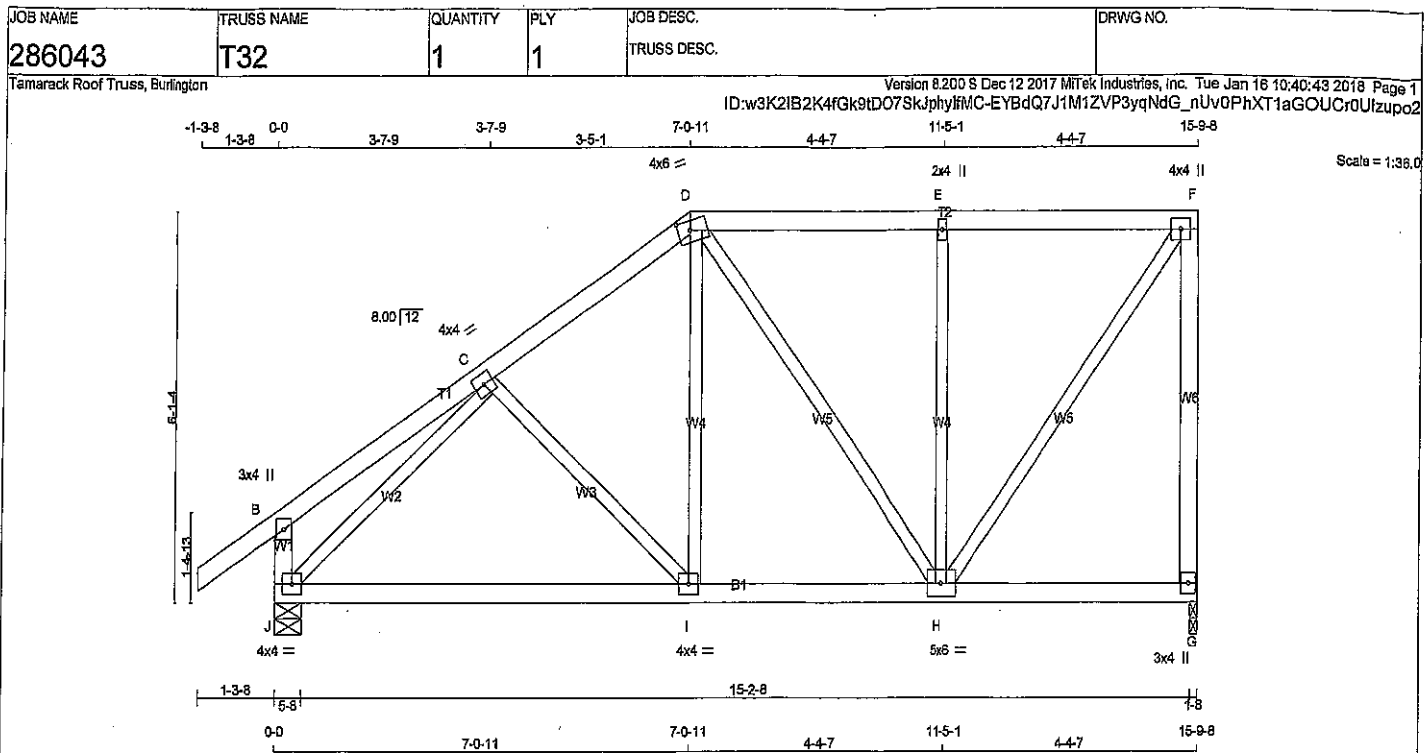
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.80 (B) (INPUT = 0.90)
JSI METAL= 0.28 (H) (INPUT = 1.00)



DRWG NO. TAM 3996-18
STRUCTURAL
COMPONENT ONLY



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
G - F	2x4	DRY	No.2
J - B	2x4	DRY	No.2
J - G	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-l	MT20	4.0	4.0		
D	TTWW-m	MT20	4.0	6.0	1.75	2.50
E	TMWW-w	MT20	2.0	4.0		
F	TMWW+p	MT20	4.0	4.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
G	886	0	886	0	1-8	1-8
J	1002	0	1002	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	WIND			
G	728	404 / 0	166 / 0	0 / 0	0 / 0	166 / 0	0 / 0
J	805	474 / 0	166 / 0	0 / 0	0 / 0	166 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 MAX		MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED UNBRAC LENGTH FR-TO	
		FROM	TO			FROM	TO
A-B	0 / 32	-84.3	-84.3 0.11 (1)	C-I	-127 / 52	0.05	(1)
B-C	0 / 16	-84.3	-84.3 0.16 (1)	I-D	0 / 348	0.08	(2)
C-D	-758 / 0	-84.3	-84.3 0.17 (1)	D-H	-169 / 0	0.17	(2)
D-E	-522 / 0	-84.3	-84.3 0.27 (1)	H-E	-453 / 0	0.27	(1)
E-F	-522 / 0	-84.3	-84.3 0.27 (1)	F-H	0 / 864	0.19	(1)
G-F	-831 / 0	0.0	0.0 0.60 (1)	J-C	-967 / 0	0.38	(1)
J-B	-231 / 0	0.0	0.0 0.02 (1)				
J-I	0 / 707	-28.0	-28.0 0.36 (2)				
I-H	0 / 619	-28.0	-28.0 0.38 (2)				
H-G	0 / 0	-28.0	-28.0 0.13 (2)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
DL =	3.0	PSF
BOT CH.	LL = 10.5	PSF
DL =	7.0	PSF
TOTAL LOAD	= 46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 089-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.53")
CALCULATED VERT. DEFL.(LL) = $L/696$ (0.11")
ALLOWABLE DEFL.(TL) = $L/360$ (0.53")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.18")

CSI: TC=0.60/1.00 (F-G:1), BC=0.38/1.00 (H-I:2), WB=0.38/1.00 (C-J:1), SSI=0.18/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

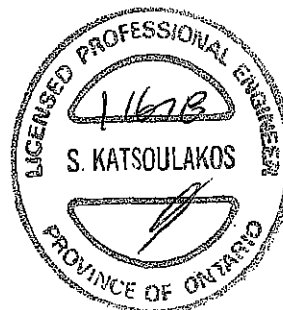
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1867
	822	2284	1658

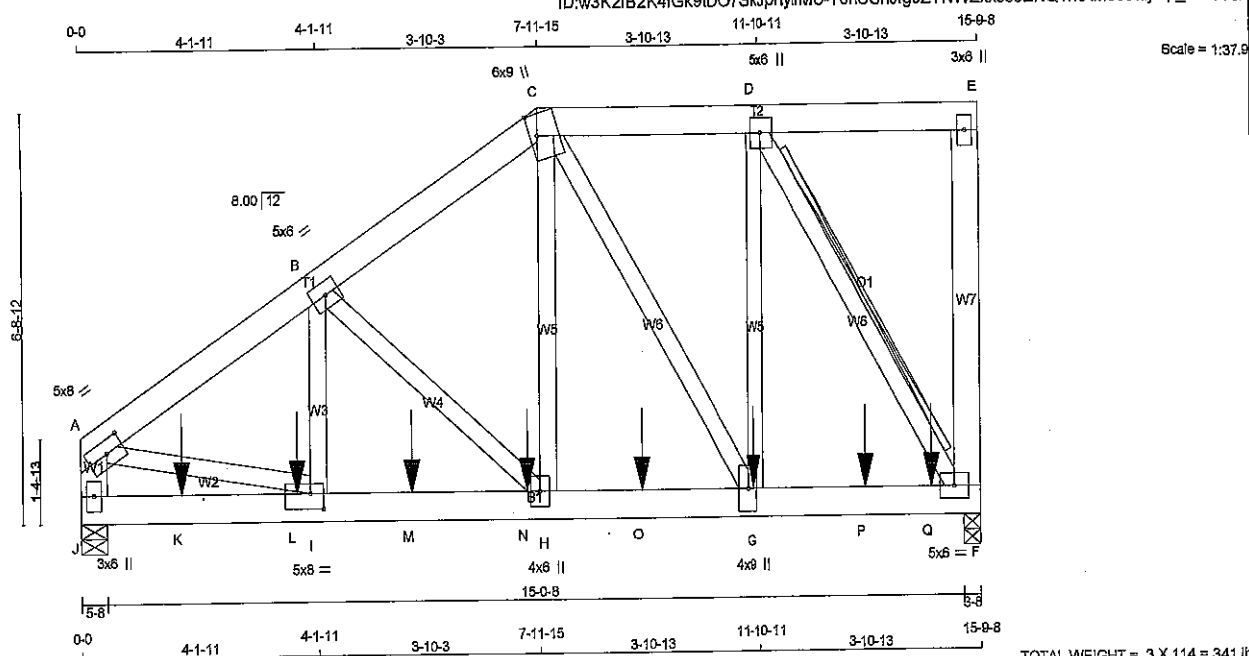
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (F) (INPUT = 0.90)
JSI METAL= 0.35 (C) (INPUT = 1.00)



DRWG NO. TAM 3997-13
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
F - E	2x6	DRY	No.2	SPF
J - A	2x6	DRY	No.2	SPF
J - F	2x6	DRY	2100F 1.8E	SPF

ALL WEBS 2x4 DRY No.2 SPF EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C 2	12	TOP
C - E 2	12	TOP
E - F 2	12	TOP
J - A 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
J - F 2	7	SIDE(893.7)
WEBS : (0.122"x3") SPIRAL NAILS		
2x4 1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0	2.50 3.75
B	TMVW-t	MT20	5.0	6.0	
C	TTVW+m	MT20	6.0	9.0	4.25 1.50
D	TMVW-t	MT20	5.0	6.0	
E	TMV+p	MT20	3.0	6.0	
F	BMVW-t	MT20	5.0	6.0	
G	BMVW-t	MT20	4.0	9.0	
H	BMVW-t	MT20	4.0	6.0	
I	BMVW-t	MT20	5.0	6.0	3.00 2.75
J	BMV-t	MT20	3.0	6.0	

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT VERT	8449	0	0	0
F	8449	0	3-8	3-8
J	7250	0	0	5-8

UNFACTORED REACTIONS

	1ST CASE	MAX / MIN	COMPONENT REACTIONS
JT COMBINED	6880	3913 / 0	1505 / 0
F	5883	3379 / 0	1264 / 0
J			

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.97 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x6 DRY SPF No.2 T-BRACE AT D-F

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	
FR-TO		FROM			FR-TO				
A-B	-8296 / 0	-84.3	-84.3	0.09 (1)	4.97	I-B	0 / 2683	0.14 (1)	
B-C	-8007 / 0	-84.3	-84.3	0.07 (1)	5.65	B-H	-2568 / 0	0.28 (1)	
C-D	-4047 / 0	-84.3	-84.3	0.03 (1)	6.25	H-C	0 / 4753	0.25 (1)	
D-E	0 / 0	-84.3	-84.3	0.03 (1)	10.00	C-G	-2035 / 0	0.46 (1)	
E-F	-141 / 0	0.0	0.0	0.02 (1)	7.81	G-D	0 / 6012	0.32 (1)	
J-A	-6285 / 0	0.0	0.0	0.13 (1)	7.01	D-F	-7548 / 0	0.48 (1)	
						A-I	0 / 7051	0.38 (1)	
J-K	0 / 0	-28.0	-28.0	0.20 (1)	10.00				
K-L	0 / 0	-28.0	-28.0	0.20 (1)	10.00				
L-I	0 / 0	-28.0	-28.0	0.20 (1)	10.00				
I-M	0 / 6891	-28.0	-28.0	0.26 (1)	10.00				
M-N	0 / 6891	-28.0	-28.0	0.26 (1)	10.00				
N-H	0 / 6891	-28.0	-28.0	0.26 (1)	10.00				
H-O	0 / 5108	-28.0	-28.0	0.23 (1)	10.00				
O-G	0 / 5108	-28.0	-28.0	0.23 (1)	10.00				
G-P	0 / 4047	-28.0	-28.0	0.29 (1)	10.00				
P-Q	0 / 4047	-28.0	-28.0	0.29 (1)	10.00				
Q-F	0 / 4047	-28.0	-28.0	0.29 (1)	10.00				

1/3/19

LICENSED PROFESSIONAL ENGINEER

S. KATSOULAKOS

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
G	11-9-4	-1712	-1712	---	FRONT	VERT	TOTAL
K	1-9-4	-1826	-1826	---	FRONT	VERT	TOTAL
L	3-9-4	-1826	-1826	---	FRONT	VERT	TOTAL
M	5-9-4	-1712	-1712	---	FRONT	VERT	TOTAL
N	7-9-4	-1712	-1712	---	FRONT	VERT	TOTAL
O	9-9-4	-1712	-1712	---	FRONT	VERT	TOTAL
P	13-9-4	-1712	-1712	---	FRONT	VERT	TOTAL
Q	14-11-4	-1714	-1714	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.8 PSF
DL = 3.0 PSF

BOT CH. LL = 10.5 PSF
DL = 7.0 PSF

TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.53")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.53")
CALCULATED VERT. DEFL.(TL) = L/999 (0.09")

CSI: TC=0.13/1.00 (A-J:1), BC=0.29/1.00 (F-G:1), WB=0.49/1.00 (D-F:1), SSI=0.58/1.00 (H-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

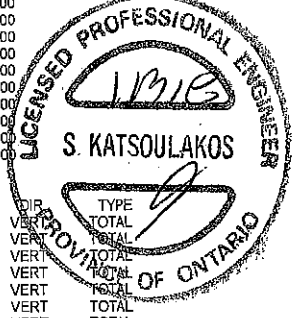
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354 1867 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (I) (INPUT = 0.90)
JSI METAL= 0.52 (A) (INPUT = 1.00)



DWG NO. TAM 3335-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285864	T33Z4	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1826.2 lbs FACTORED DOWN AT 1-9-4, 1826.2 lbs FACTORED DOWN AT 3-9-4, 1711.9 lbs FACTORED DOWN AT 5-9-4, 1711.9 lbs FACTORED DOWN AT 7-9-4, 1711.9 lbs FACTORED DOWN AT 9-9-4, 1711.9 lbs FACTORED DOWN AT 11-9-4, AND 1711.9 lbs FACTORED DOWN AT 13-9-4, AND 1714.0 lbs FACTORED DOWN AT 14-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

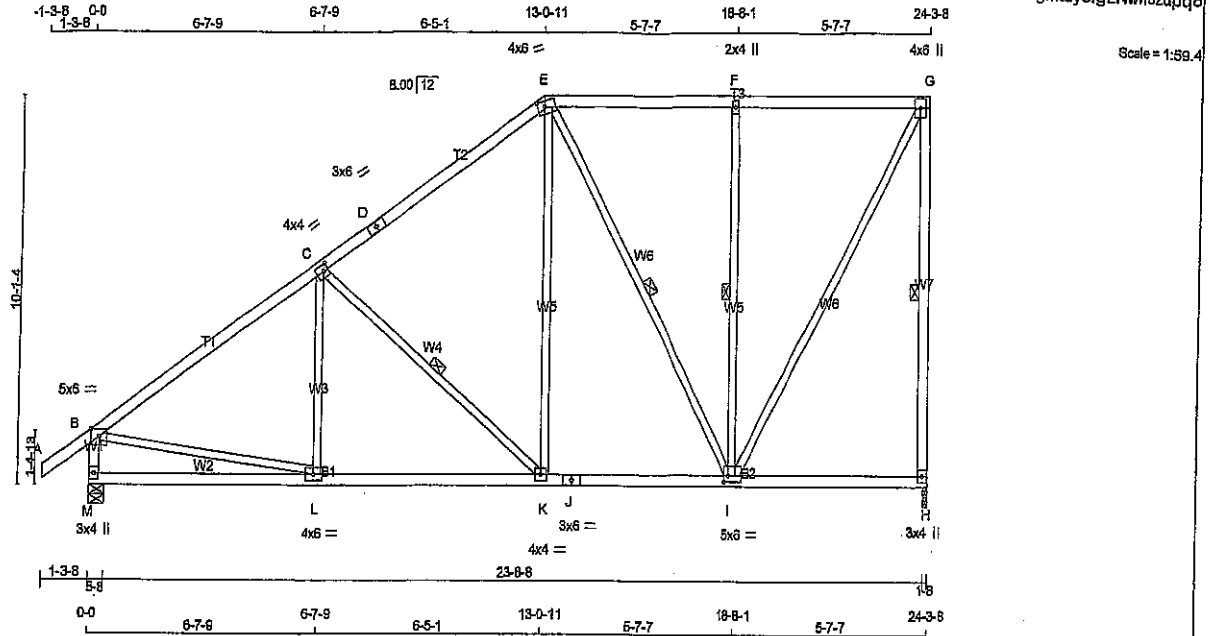


DWG NO. TAM 3335-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43554	DRWG NO.
286039	T37	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.200 S Dec 12 2017 Mitek Industries, Inc. Tue Jan 16 10:38:29 2018 Page 1
ID:w3K2IB2K4fGk9ID07SkJphyfMC-9n5zLjI_wsgWKT4XUsZLEAM5TgmdtyefgLNwn3zupq8



TOTAL WEIGHT = 2 X 122 = 243 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF		
D - E	2x4	DRY	No.2	SPF		
E - G	2x4	DRY	No.2	SPF		
H - G	2x4	DRY	No.2	SPF		
M - B	2x4	DRY	No.2	SPF		
M - J	2x4	DRY	No.2	SPF		
J - H	2x4	DRY	No.2	SPF		

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTVW-m	MT20	4.0	6.0	1.75	2.50
F	TMVW-w	MT20	2.0	4.0		
G	TMVW+p	MT20	4.0	6.0		
H	BMV1-p	MT20	3.0	4.0		
I	BMVWV-t	MT20	5.0	6.0	2.00	1.50
J	BS-t	MT20	3.0	6.0		
K	BMVW-t	MT20	4.0	4.0		
L	BMVW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	HORIZ	IN-SX
H	1383	0	1383	0
M	1479	0	1479	0

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN	COMPONENT REACTIONS
	COMBINED	SNOW	LIVE
H	1119	621 / 0	255 / 0
M	1198	692 / 0	255 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.69 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-H, C-K, E-I, F-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE	VERT. LOAD LC1	MEMB.	FORCE
	(LBS)	(PLF)		(LBS)
FR-TO		FROM TO	FR-TO	
A-B	0 / 32	-84.3 -84.3 0.11 (1)	L-C	-1 / 267
B-C	-1522 / 0	-84.3 -84.3 0.55 (1)	C-K	-594 / 0
C-D	-1051 / 0	-84.3 -84.3 0.49 (1)	K-E	0 / 545
D-E	-1051 / 0	-84.3 -84.3 0.49 (1)	E-I	-413 / 0
E-F	-844 / 0	-84.3 -84.3 0.34 (1)	I-G	-586 / 0
F-G	-644 / 0	-84.3 -84.3 0.34 (1)	G-L	0 / 1289
H-G	-1259 / 0	0.0 0.0 0.85 (1)	B-L	0 / 1314
M-B	-1404 / 0	0.0 0.0 0.14 (1)		
M-L	0 / 0	-28.0 -28.0 0.31 (3)		
L-K	0 / 1296	-28.0 -28.0 0.46 (2)		
K-J	0 / 845	-28.0 -28.0 0.31 (2)		
J-I	0 / 845	-28.0 -28.0 0.31 (2)		
I-H	0 / 0	-28.0 -28.0 0.21 (3)		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 25.8	PSF
	DL	= 3.0	PSF
BOT CH.	LL	= 10.5	PSF
	DL	= 7.0	PSF
TOTAL LOAD		= 46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL)= L/999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL)= L/999 (0.14")

CSI: TC=0.65/1.00 (G-H:1), BC=0.46/1.00 (K-L:2), WB=0.41/1.00 (F-I:1), SSI=0.23/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.80

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

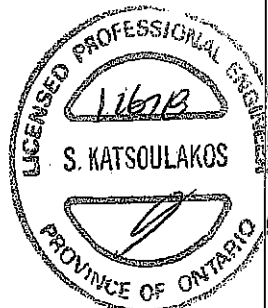
NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 622

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.83 (L) (INPUT = 0.90)
SI METAL= 0.32 (L) (INPUT = 1.00)

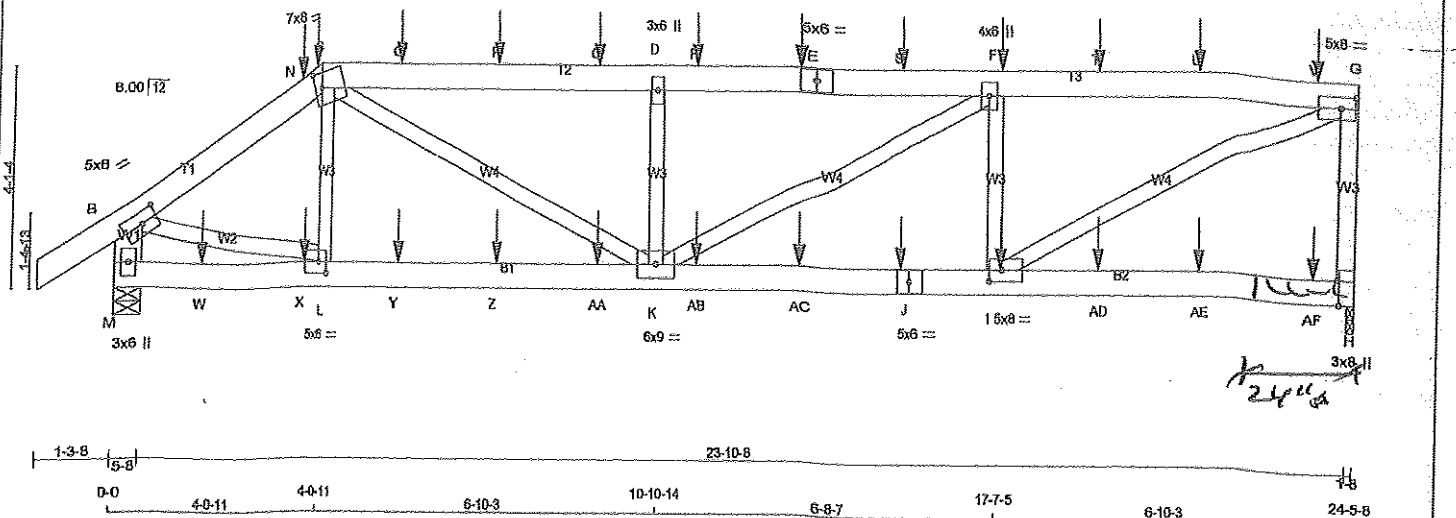


DRWG NO. TAM 3894 -13
STRUCTURAL
COMPONENT ONLY

JOB NAME: 285956 TRUSS NAME: T40 QUANTITY: 1 PLY: 1 JOB DESC.: TRUSS DESC. DRWG NO. Tamarack Roof Truss, Burlington

Version 8.206 5 Dec 12 2017 Mitek Industries, Inc. Thu Jan 25 08:19:26 2018 Page 1
ID: w3K2IB2K4fGk9DO7SkJphylfMC-4B5sUzFup8yvMnC71ILAK_9gHr2QLIFZ2370sCzru0V

Scale = 1:42.0



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x6	DRY No.2	SPF
C - E	2x6	DRY No.2	SPF
E - G	2x6	DRY No.2	SPF
H - G	2x4	DRY No.2	SPF
M - B	2x6	DRY No.2	SPF
M - J	2x6	DRY No.2	SPF
J - H	2x6	DRY No.2	SPF
ALL WEBS EXCEPT	2x4	DRY No.2	SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT 2309	DOWN 2309	0	1-8
M	VERT 2439	DOWN 2439	0	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED SNOW LIVE PERM. LIVE WIND DEAD SOIL
H	1907 1040 / 0 447 / 0 0 / 0 0 / 0 420 / 0 0 / 0
M	1982 1134 / 0 429 / 0 0 / 0 0 / 0 420 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 240 IN. C/C

LOADING IN PLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = 1/360 (0.02")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.15")
ALLOWABLE DEFL.(TL) = 1/360 (0.02")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.24")

CSI: TC=0.58/1.00 (C-D:1), EC=0.57/1.00 (H-K:2), WB=0.66/1.00 (G-I:1), SS=0.35/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PL) (PLI)
MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.62 (G) (INPUT = 1.00)

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMVW-1	MT20	5.0	8.0	2.50 3.75
C	TMVW-m	MT20	7.0	8.0	3.25 2.00
D	TMVW-w	MT20	3.0	6.0	
E	TS-1	MT20	5.0	6.0	
F	TMVW-t	MT20	4.0	6.0	
G	TMVW-1	MT20	5.0	8.0	2.50 3.00
H	BMV1+p	MT20	3.0	8.0	Edge 0.50
I	BMVW-1	MT20	5.0	8.0	2.50 3.00
J	BS-1	MT20	5.0	6.0	
K	BMVW-w-1	MT20	0.0	0.0	
L	BMVW-1	MT20	5.0	6.0	2.50 2.25
M	BMV1+p	MT20	3.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 244.5 lbs FACTORED DOWN AT 4-0-11, 129.8 lbs FACTORED DOWN AT 3-6-12, 101.4 lbs FACTORED DOWN AT 5-8-12, 101.4 lbs FACTORED DOWN AT 7-8-12, 101.4 lbs FACTORED DOWN AT 9-8-12, 101.4 lbs FACTORED DOWN AT 11-8-12, 101.4 lbs FACTORED DOWN AT 13-8-12, 101.4 lbs FACTORED DOWN AT 15-8-12, 101.4 lbs FACTORED DOWN AT 17-8-12, 101.4 lbs FACTORED DOWN AT 19-8-12, AND 101.4 lbs FACTORED DOWN AT 21-8-12, AND 112.8 lbs FACTORED DOWN AT 23-8-12 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 1-8-12, 69.9 lbs FACTORED DOWN AT 3-8-12, 69.9 lbs FACTORED DOWN AT 5-8-12, 69.9 lbs FACTORED DOWN AT 7-8-12, 69.9 lbs FACTORED DOWN AT 9-8-12, 69.9 lbs FACTORED DOWN AT 11-8-12, 69.9 lbs FACTORED DOWN AT 13-8-12, 69.9 lbs FACTORED DOWN AT 15-8-12, 69.9 lbs FACTORED DOWN AT 17-8-12, 69.9 lbs FACTORED DOWN AT 19-8-12, AND 69.9 lbs FACTORED DOWN AT 21-8-12, AND 76.6 lbs FACTORED DOWN AT 23-8-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.72 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	
FR-TO				FR-TO			
A-B	0/33	-84.3	0.07 (1)	10.00	L-C	-197 / 285	
B-N	-2810 / 0	-84.3	0.19 (1)	4.80	C-K	0 / 1888	
N-C	-2810 / 0	-84.3	0.19 (1)	4.80	K-D	-972 / 0	
C-O	-4019 / 0	-84.3	0.56 (1)	3.72	K-F	0 / 816	
O-P	-4018 / 0	-84.3	0.56 (1)	3.72	F-F	-1395 / 0	
P-Q	-4018 / 0	-84.3	0.56 (1)	3.72	I-G	0 / 3739	
Q-D	-4018 / 0	-84.3	0.56 (1)	3.72	B-L	0 / 2410	
D-R	-4018 / 0	-84.3	0.56 (1)	3.72			
R-E	-4018 / 0	-84.3	0.56 (1)	3.72			
E-S	-4018 / 0	-84.3	0.56 (1)	3.72			
S-F	-4018 / 0	-84.3	0.56 (1)	3.72			
F-T	-3301 / 0	-84.3	0.53 (1)	4.08			
T-U	-3301 / 0	-84.3	0.53 (1)	4.08			
U-V	-3301 / 0	-84.3	0.53 (1)	4.08			
V-G	-3301 / 0	-84.3	0.53 (1)	4.08			
H-G	-2160 / 0	0.0	0.0	5.72			
M-B	-2388 / 0	0.0	0.0	1.71 (1)			

M-W	0 / 0	-28.0	-28.0	0.19 (2)	10.00
W-X	0 / 0	-28.0	-28.0	0.19 (2)	10.00
X-L	0 / 0	-28.0	-28.0	0.19 (2)	10.00
L-Y	0 / 2349	-28.0	-28.0	0.42 (1)	10.00
Y-Z	0 / 2349	-28.0	-28.0	0.42 (1)	10.00
Z-AA	0 / 2349	-28.0	-28.0	0.42 (1)	10.00
AA-K	0 / 2349	-28.0	-28.0	0.42 (1)	10.00
K-AB	0 / 3301	-28.0	-28.0	0.57 (2)	10.00
AB-AC	0 / 3301	-28.0	-28.0	0.57 (2)	10.00
AC-J	0 / 3301	-28.0	-28.0	0.57 (2)	10.00
J-I	0 / 3301	-28.0	-28.0	0.57 (2)	10.00
I-AD	0 / 0	-28.0	-28.0	0.28 (3)	10.00
AD-AE	0 / 0	-28.0	-28.0	0.26 (3)	10.00
AE-AF	0 / 0	-28.0	-28.0	0.26 (3)	10.00
AF-H	0 / 0	-28.0	-28.0	0.26 (3)	10.00

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE
C	4-0-11	-23	-25	FRONT	VERT	DEAD
C	4-0-11	-222	-222	FRONT	VERT	SNOW
E	13-8-12	-101	-101	BACK	VERT	TOTAL

PROFESSIONAL ENGINEER
S. KATSOUAKOS
PROVINCE OF ONTARIO

DRWG NO. TAM5902-18
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2

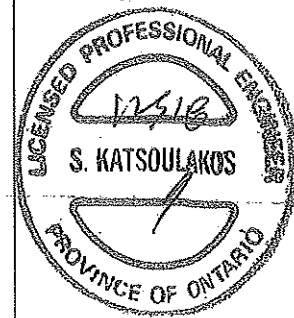
JOB NAME 285956	TRUSS NAME T40	QUANTITY 1	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.200 S Dec 12 2017 Mitek Industries, Inc. Thu Jan 25 08:19:26 2018 Page 2
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FACTORED CONCENTRATED LOADS (LBS)						
JT	LCC	LC1	MAX	MAX*	FACE	DIR. TYPE
F	17-8-12	-101	-101	---	BACK	VERT TOTAL
I	17-8-12	-40	-70	---	BACK	VERT TOTAL
J	15-8-12	-40	-70	---	BACK	VERT TOTAL
N	3-8-12	-130	-130	---	BACK	VERT TOTAL
O	5-8-12	-101	-101	---	BACK	VERT TOTAL
P	7-8-12	-101	-101	---	BACK	VERT TOTAL
Q	9-8-12	-101	-101	---	BACK	VERT TOTAL
R	11-8-12	-101	-101	---	BACK	VERT TOTAL
S	15-8-12	-101	-101	---	BACK	VERT TOTAL
T	19-8-12	-101	-101	---	BACK	VERT TOTAL
U	21-8-12	-101	-101	---	BACK	VERT TOTAL
V	23-8-12	-113	-113	---	BACK	VERT TOTAL
W	1-8-12	-40	-70	---	BACK	VERT TOTAL
X	3-8-12	-40	-70	---	BACK	VERT TOTAL
Y	5-8-12	-40	-70	---	BACK	VERT TOTAL
Z	7-8-12	-40	-70	---	BACK	VERT TOTAL
AA	9-8-12	-40	-70	---	BACK	VERT TOTAL
AB	11-8-12	-40	-70	---	BACK	VERT TOTAL
AC	13-8-12	-40	-70	---	BACK	VERT TOTAL
AD	19-8-12	-40	-70	---	BACK	VERT TOTAL
AE	21-8-12	-40	-70	---	BACK	VERT TOTAL
AF	23-8-12	-44	-77	---	BACK	VERT TOTAL

SCAB JOINT # 4 WITH 2 X 6 #2
SPF 24" LONG 1 SIDE(S) USING 2
ROW(S) 3/4" COMMON WIRE NAILS
AT 2 " O/C STAGGERED (18 NAILS/SCAB)
REQUIRED FOR 1 1/2 " BRG PROVISION.

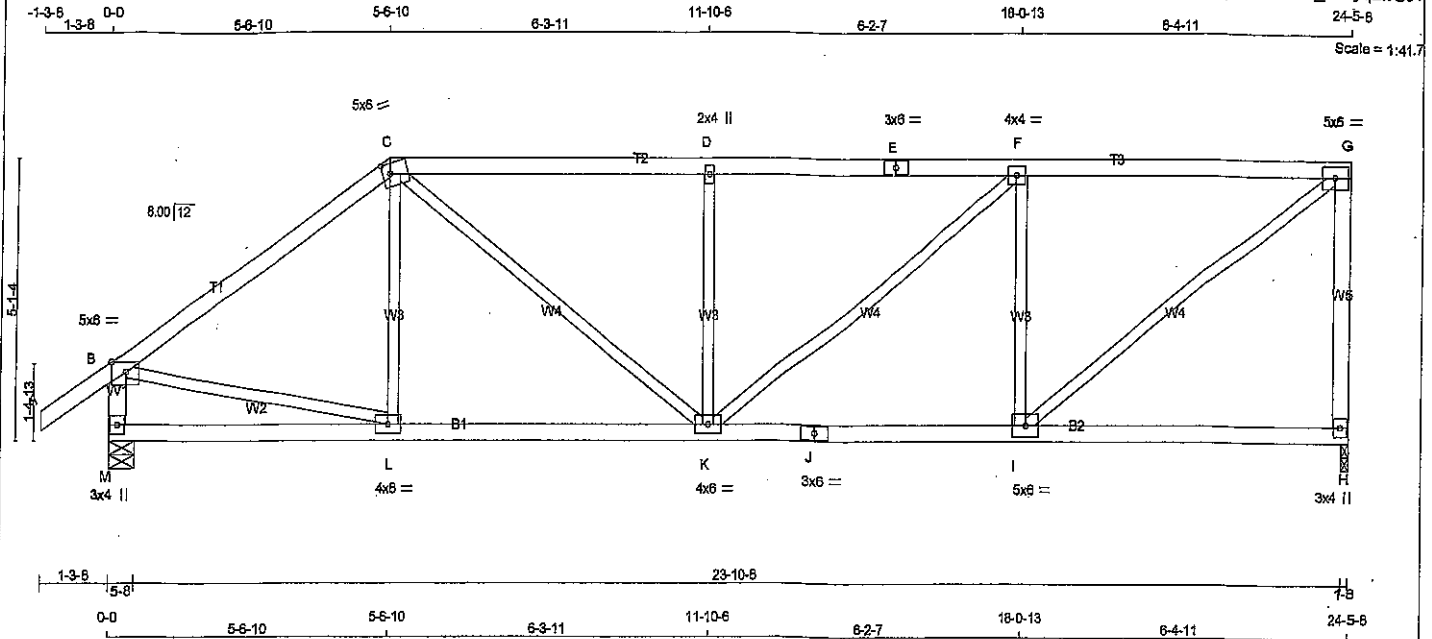


DWG NO. TAM 5982-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285815	T41	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 \$ Dec 12 2017 Mitek Industries, Inc. Thu Jan 11 11:12:44 2018 Page 1
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TOTAL WEIGHT = 98 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
H - G	2x4	DRY	No.2
M - B	2x4	DRY	No.2
M - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
SPF			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-p	MT20	5.0	6.0	Edge
C	TTWW-m	MT20	5.0	6.0	2.25 1.75
D	TMW+w	MT20	2.0	4.0	
E	TS-t	MT20	3.0	6.0	
F	TMWW-t	MT20	4.0	4.0	
G	TMVW-t	MT20	5.0	6.0	
H	BMVt+p	MT20	3.0	4.0	
I	BMWW-t	MT20	5.0	6.0	
J	BS-t	MT20	3.0	6.0	
K	BMWWW-t	MT20	4.0	6.0	
L	BMWW-t	MT20	4.0	6.0	
M	BMVt+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	HORZ	BRG	BRG	IN-SX
H	1373	0	0	1373	0	0	1-8	1-8	
M	1488	0	0	1488	0	0	5-8	5-8	

UNFACTORED REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1127	626 / 0	257 / 0	0 / 0	0 / 0	245 / 0	0 / 0
M	1208	696 / 0	257 / 0	0 / 0	0 / 0	253 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.30 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 32	-84.3 -84.3	0.11 (1)	10.00	L-C	-50 / 204	0.05 (3)
B-C	-1507 / 0	-84.3 -84.3	0.55 (1)	4.68	B-L	0 / 1275	0.28 (1)
C-D	-1622 / 0	-84.3 -84.3	0.56 (1)	4.33	I-G	0 / 1799	0.40 (1)
D-E	-1622 / 0	-84.3 -84.3	0.56 (1)	4.30	C-K	0 / 717	0.18 (1)
E-F	-1622 / 0	-84.3 -84.3	0.56 (1)	4.30	I-F	-882 / 0	0.34 (1)
F-G	-1437 / 0	-84.3 -84.3	0.54 (1)	4.77	K-D	-571 / 0	0.22 (1)
H-G	-1300 / 0	0.0 0.0	0.57 (1)	7.08	K-F	0 / 467	0.11 (1)
M-B	-1427 / 0	0.0 0.0	0.15 (1)	6.84			
M-L	0 / 0	-28.0 -28.0	0.24 (3)	10.00			
L-K	0 / 1251	-28.0 -28.0	0.39 (2)	10.00			
K-J	0 / 1437	-28.0 -28.0	0.44 (2)	10.00			
J-I	0 / 1437	-28.0 -28.0	0.44 (2)	10.00			
I-H	0 / 0	-28.0 -28.0	0.28 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.62")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.09")
ALLOWABLE DEFL.(TL) = $L/360$ (0.62")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.15")

CSI: TC=0.58/1.00 (D-F:1), BC=0.44/1.00 (I-K:2), WB=0.40/1.00 (G-I:1), SSI=0.25/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 622 2284 1655

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)

JSI METAL= 0.44 (J) (INPUT = 1.00)

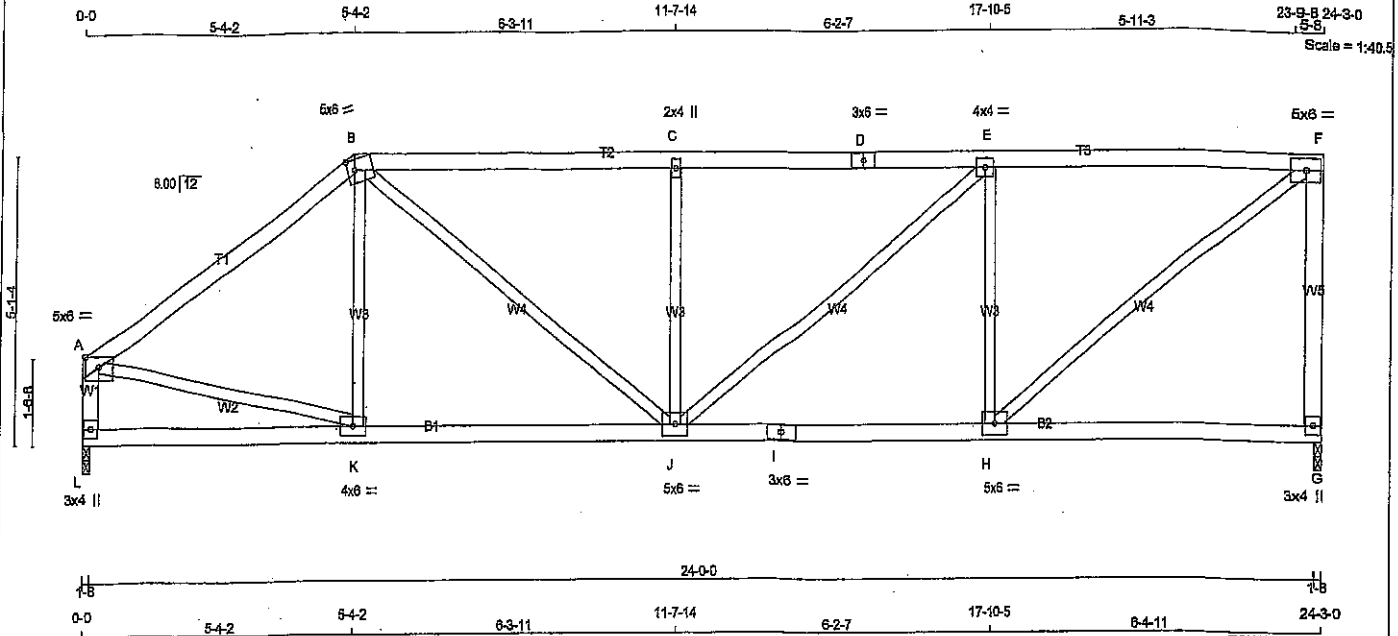


DWG NO. TAM3363.18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285815	T41A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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LUMBER	DESCR.
N. L. G. A. RULES	
CHORDS	SIZE
A - B	2x4 DRY No.2
B - D	2x4 DRY No.2
D - F	2x4 DRY No.2
G - F	2x4 DRY No.2
L - A	2x4 DRY No.2
L - I	2x4 DRY No.2
L - G	2x4 DRY No.2
ALL WEBS	2x3 DRY No.2
EXCEPT	
DRY: SEASONED LUMBER.	

PLATES (table in inches)	W	LEN	Y	X
JT TYPE	PLATES			
A - B	MT20	5.0	6.0	Edge
B - D	MT20	5.0	6.0	2.25 1.75
C - D	MT20	2.0	4.0	
D - TS-1	MT20	3.0	6.0	
E - TMWV-t	MT20	4.0	4.0	
F - TMWV-t	MT20	5.0	6.0	
G - BMV1+p	MT20	3.0	4.0	
H - BMWV-t	MT20	5.0	6.0	
I - BS-1	MT20	3.0	6.0	
J - BMWVW-t	MT20	5.0	6.0	
K - BMWV-t	MT20	4.0	6.0	
L - BMV1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	DOWN	IN-SX	IN-SX
G	1361 0	1361 0	1-8	1-8
L	1361 0	1361 0	1-8	1-8

UNFACTORED REACTIONS

1ST LCASE	MAX /MIN	COMPONENT REACTIONS
JT COMBINED	SNOW	LIVE
G	1118 620 / 0	255 / 0
L	1118 620 / 0	255 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.33 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMS.	FORCE	VERT. LOAD LC1	MEMS.	FORCE
(LBS)	(PLF)	CSI (LC)	(LBS)	CSI (LC)
FR-TO	FROM	TO	FR-TO	FROM
A-B	-1451 / 0	-84.3 -84.3 0.50 (1)	K-B	-85 / 181
B-C	-1791 / 0	-84.3 -84.3 0.56 (1)	A-K	0 / 1236
C-D	-1792 / 0	-84.3 -84.3 0.57 (1)	H-F	0 / 1780
D-E	-1792 / 0	-84.3 -84.3 0.57 (1)	B-J	0 / 798
E-F	-1422 / 0	-84.3 -84.3 0.54 (1)	H-E	-870 / 0
G-F	-1289 / 0	0.0 0.0 0.57 (1)	J-C	-571 / 0
L-A	-1302 / 0	0.0 0.0 0.14 (1)	J-E	0 / 468
L-K	0 / 0	-28.0 -28.0 0.23 (3)		
K-J	0 / 1204	-28.0 -28.0 0.38 (2)		
J-I	0 / 1422	-28.0 -28.0 0.44 (2)		
I-H	0 / 1422	-28.0 -28.0 0.44 (2)		
H-G	0 / 0	-28.0 -28.0 0.28 (3)		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
DL = 3.0 PSF	
BOT CH.	LL = 10.6 PSF
DL = 7.0 PSF	
TOTAL LOAD = 46.1 PSF	

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(65 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.81")
CALCULATED VERT. DEFL.(LL) = $L/899$ (0.09")
ALLOWABLE DEFL.(TL) = $L/360$ (0.81")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.14")

CSI: TC=0.57/1.00 (C-E-1), BC=0.44/1.00 (H-J-2), WB=0.40/1.00 (F-H-1), SS=0.25/1.00 (E-F-1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	816	354	1687
	822	2284	1655

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.85 (K) (INPUT = 0.90)
JSI METAL= 0.44 (I) (INPUT = 1.00)



DRWG NO. TAM 5072-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285815	T42A	1	1	TRUSS DESC.	

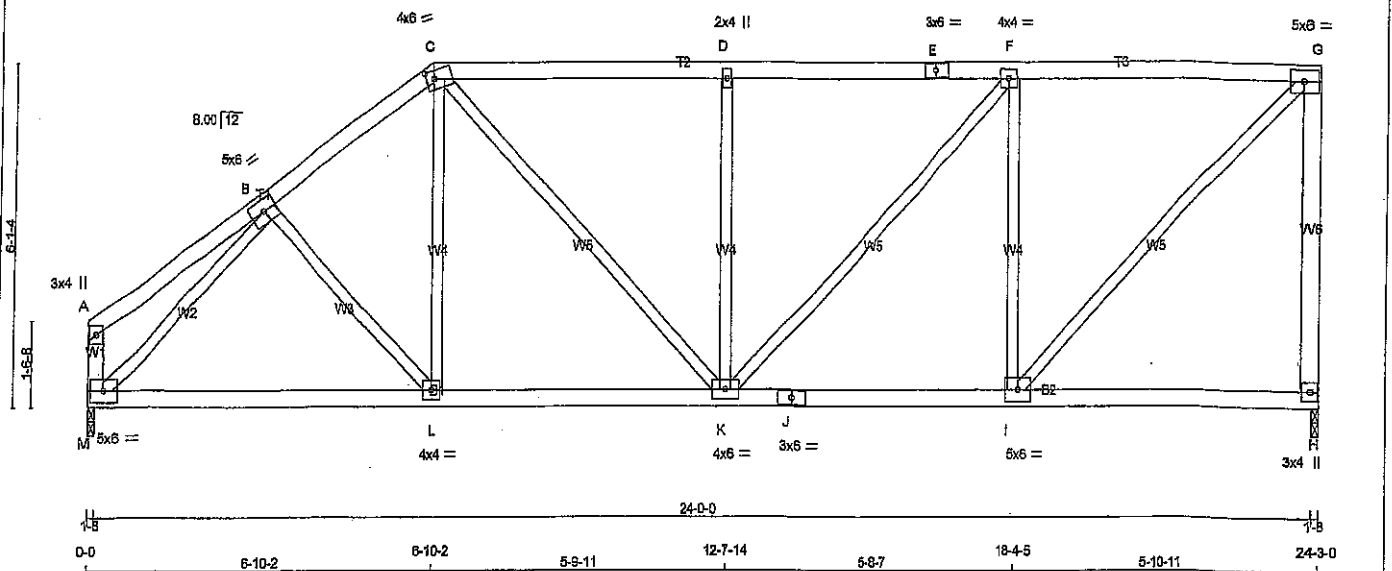
Tamarack Roof Truss, Burlington

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0-03-63-53-135-10-25-9-1112-7-145-5-719-4-55-5-323-9-824-3-05-8

Scale = 1:40.7



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	DRY
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
H - G	2x4	DRY	No.2
M - A	2x4	DRY	No.2
M - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
SPF			
DRY: SEASONED LUMBER.			

PLATES (table in inches)				
JT	TYPE	PLATES	W	LEN Y X
A	TMV+p	MT20	3.0	4.0
B	TMWW-t	MT20	5.0	8.0
C	TTWW-m	MT20	4.0	8.0 1.75 1.75
D	TMWW-w	MT20	2.0	4.0
E	TS-t	MT20	3.0	6.0
F	TMWW-t	MT20	4.0	4.0
G	TMWW-t	MT20	5.0	6.0
H	BMV1+p	MT20	3.0	4.0
I	BMWW-t	MT20	5.0	8.0
J	BS-t	MT20	3.0	6.0
K	BMWW-t	MT20	4.0	6.0
L	BMWW-t	MT20	4.0	4.0
M	BMVW1-t	MT20	5.0	6.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQD	
GROSS REACTION		GROSS REACTION		BRG		BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	1361	0	1361	0	0	1-8	1-8
M	1361	0	1361	0	0	1-8	1-8

UNFACTORED REACTIONS		MAX/MIN COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE
H	1116	620 / 0	255 / 0
M	1116	620 / 0	255 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.85 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX	MAX. MEMB.	FORCE (LBS)	MAX. MEMB.	FORCE (LBS)
FR-TO		FROM	TO	LENGTH	FR-TO		
A-B	0 / 17	-84.3	-84.3 0.14 (1)	10.00	B-L	0 / 134	0.03 (3)
B-C	-1430 / 0	-84.3	-84.3 0.17 (1)	5.28	L-C	0 / 278	0.06 (2)
C-D	-1471 / 0	-84.3	-84.3 0.44 (1)	4.68	M-B	-1633 / 0	0.62 (1)
D-E	-1471 / 0	-84.3	-84.3 0.45 (1)	4.85	I-G	0 / 1556	0.35 (1)
E-F	-1471 / 0	-84.3	-84.3 0.45 (1)	4.85	C-K	0 / 415	0.09 (1)
F-G	-1109 / 0	-84.3	-84.3 0.43 (1)	5.43	I-F	-906 / 0	0.53 (1)
H-G	-1295 / 0	0.0	0.0 0.93 (1)	7.10	K-D	-525 / 0	0.31 (1)
M-A	-114 / 0	0.0	0.0 0.01 (1)	7.81	K-F	0 / 517	0.12 (1)
M-L	0 / 1141	-28.0	-28.0 0.43 (2)	10.00			
L-K	0 / 1177	-28.0	-28.0 0.45 (2)	10.00			
K-J	0 / 1109	-28.0	-28.0 0.37 (2)	10.00			
J-I	0 / 1109	-28.0	-28.0 0.37 (2)	10.00			
I-H	0 / 0	-28.0	-28.0 0.24 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN./OC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TFC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.93/1.00 (G-H:1), BC=0.45/1.00 (K-L:2),
WB=0.62/1.00 (B-M:1), SS=0.23/1.00 (F-G:1)

DOL LUMBER=1.00 NAL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 822 2284 1856

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.86 (C) (INPUT = 0.80)
JSI METAL = 0.40 (B) (INPUT = 1.00)



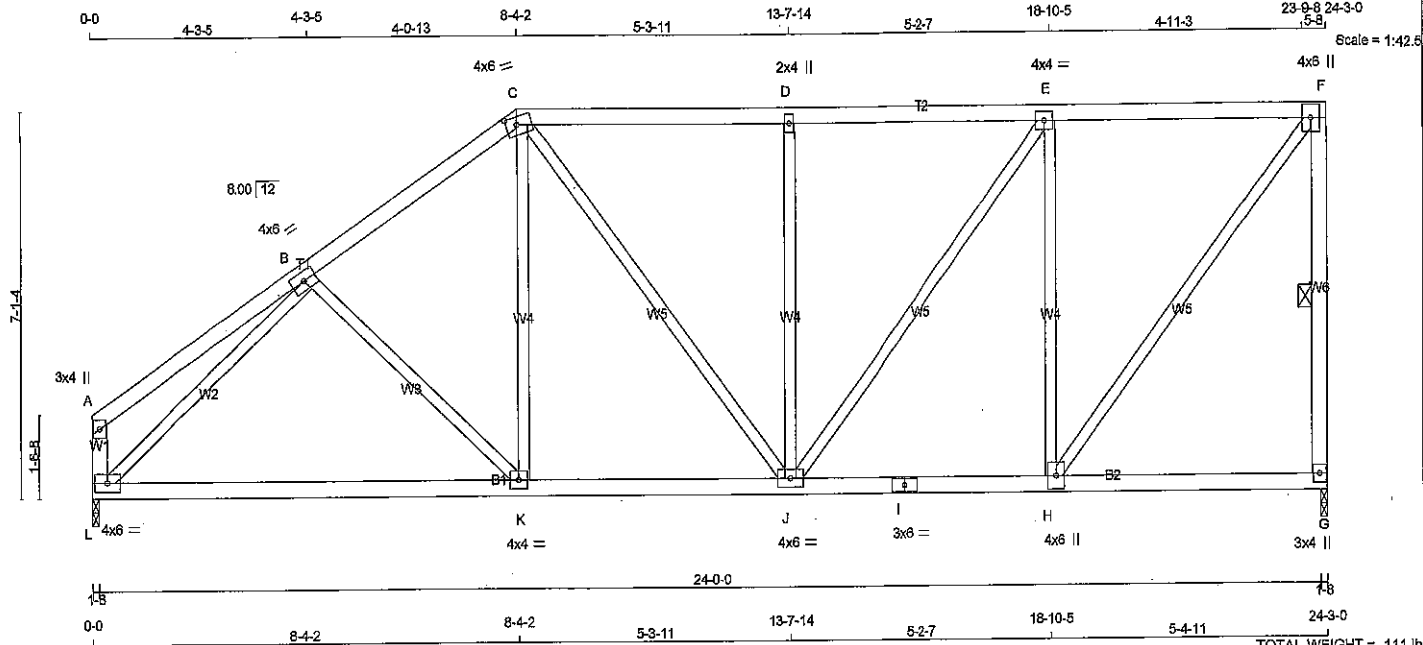
DWG NO. TAM 5073 -18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285815	T43A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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LUMBER	N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS				
A - C	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - A	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
DRY: SEASONED LUMBER.				

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
G	1361	0	1361	0
L	1361	0	1361	0

UNFACTORED REACTIONS

1ST CASE	MAX./MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED				
G	1118	620 / 0	255 / 0	0 / 0	243 / 0
L	1118	620 / 0	255 / 0	0 / 0	243 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. VERT. LOAD (LC1)	MAX. UNBRACED LENGTH (FT)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. VERT. LOAD (LC)
FR-TO					FR-TO		
A-B	0 / 22	-84.3	-84.3	0.23 (1)	10.00	B-K	-77 / 99
B-C	-1369 / 0	-84.3	-84.3	0.27 (1)	5.25	K-C	0 / 387
C-D	-1227 / 0	-84.3	-84.3	0.35 (1)	5.36	L-B	-1629 / 0
D-E	-1227 / 0	-84.3	-84.3	0.36 (1)	5.33	H-F	0 / 1425
E-F	-884 / 0	-84.3	-84.3	0.34 (1)	6.05	C-J	0 / 188
G-F	-1302 / 0	0.0	0.0	0.30 (1)	5.66	H-E	-940 / 0
L-A	-136 / 0	0.0	0.0	0.01 (1)	7.81	J-D	-479 / 0
						J-E	0 / 566
L-K	0 / 1176	-28.0	-28.0	0.56 (2)	10.00		
K-J	0 / 1124	-28.0	-28.0	0.57 (2)	10.00		
J-I	0 / 884	-28.0	-28.0	0.32 (2)	10.00		
I-H	0 / 884	-28.0	-28.0	0.32 (2)	10.00		
H-G	0 / 0	-28.0	-28.0	0.21 (3)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, 50BC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/996 (0.21")
ALLOWABLE DEFL.(TL) = L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/853 (0.34")

CSI: TC=0.36/1.00 (D-E-1), BC=0.57/1.00 (J-K-2), WB=0.90/1.00 (B-L-1), SSI=0.21/1.00 (E-F-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1887 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.40 (B) (INPUT = 1.00)



DRWG NO. TAM 5074 -8
STRUCTURAL
COMPONENT ONLY



DESCR.
SPF
SPF
SPF
SPF
SPF
SPF

SPF
SPF
SPF

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

BUILDING BEARINGS

UNFACTORED REACTIONS

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.74 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-F, C-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	-1477 / 0	-84.3 -84.3	0.40 (1)	4.93	J-B	-158 / 110	0.06 (1)
B-C	-1264 / 0	-84.3 -84.3	0.38 (1)	5.24	B-I	-307 / 0	0.26 (1)
C-D	-973 / 0	-84.3 -84.3	0.83 (1)	4.74	I-C	0 / 419	0.09 (2)
D-E	-973 / 0	-84.3 -84.3	0.63 (1)	4.74	C-G	-87 / 0	0.05 (1)
E-F	-1281 / 0	0.0 0.0	0.38 (1)	5.70	G-D	-754 / 0	0.95 (1)
K-A	-1301 / 0	0.0 0.0	0.14 (1)	7.08	G-E	0 / 1431	0.23 (1)
					A-J	0 / 1288	0.29 (1)
K-J	0 / 0	-28.0 -28.0	0.15 (3)	10.00			
J-I	0 / 1252	-28.0 -28.0	0.33 (2)	10.00			
I-H	0 / 1032	-28.0 -28.0	0.50 (2)	10.00			
H-G	0 / 1032	-28.0 -28.0	0.50 (2)	10.00			
G-F	0 / 0	-28.0 -28.0	0.38 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.83/1.00 (D-E:1), BC=0.50/1.00 (G-I:2),
WB=0.95/1.00 (D-G:1), SS=0.30/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

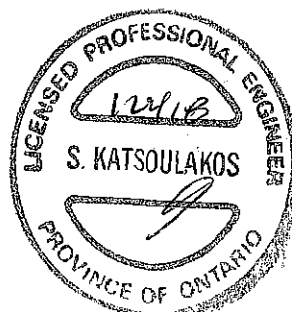
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES						
PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	622	2284	1650

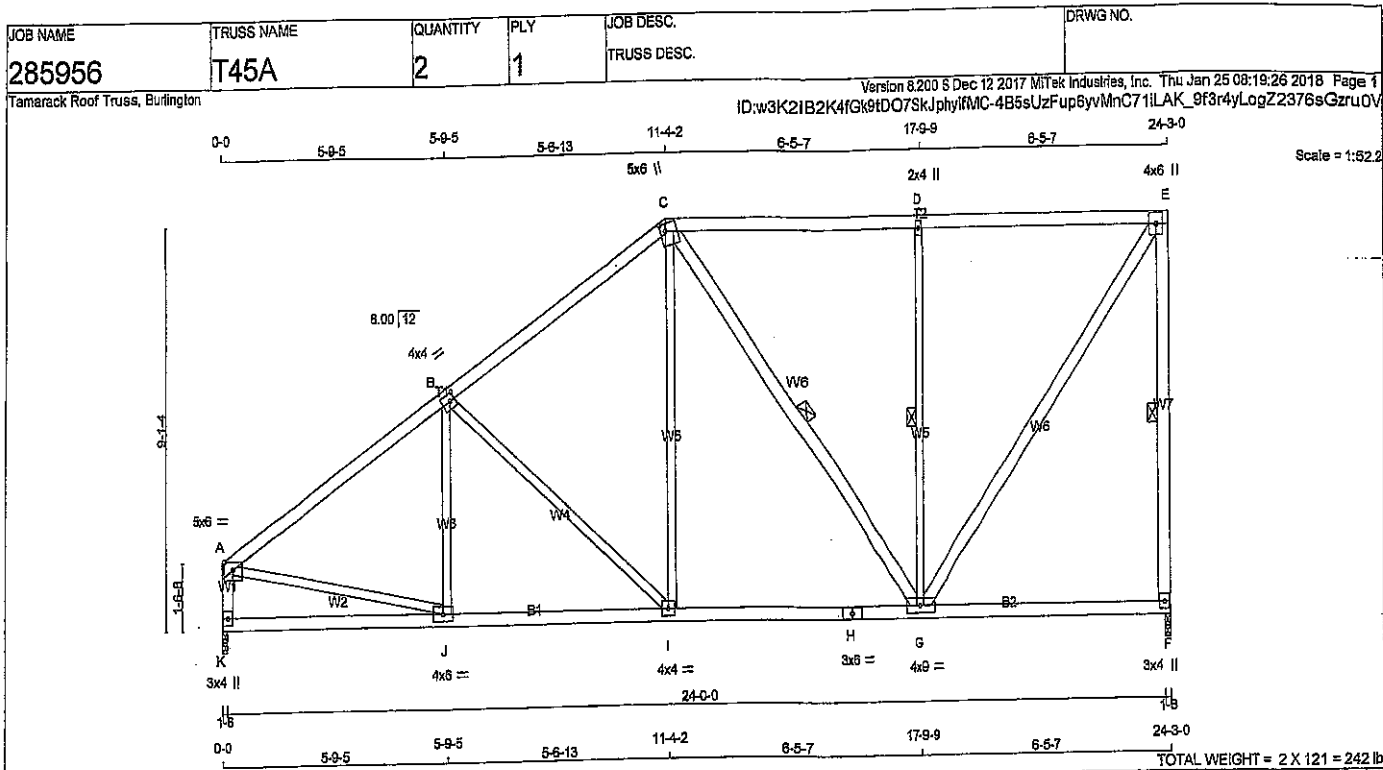
PLATE PLACEMENT TOL. = 0.250 inches

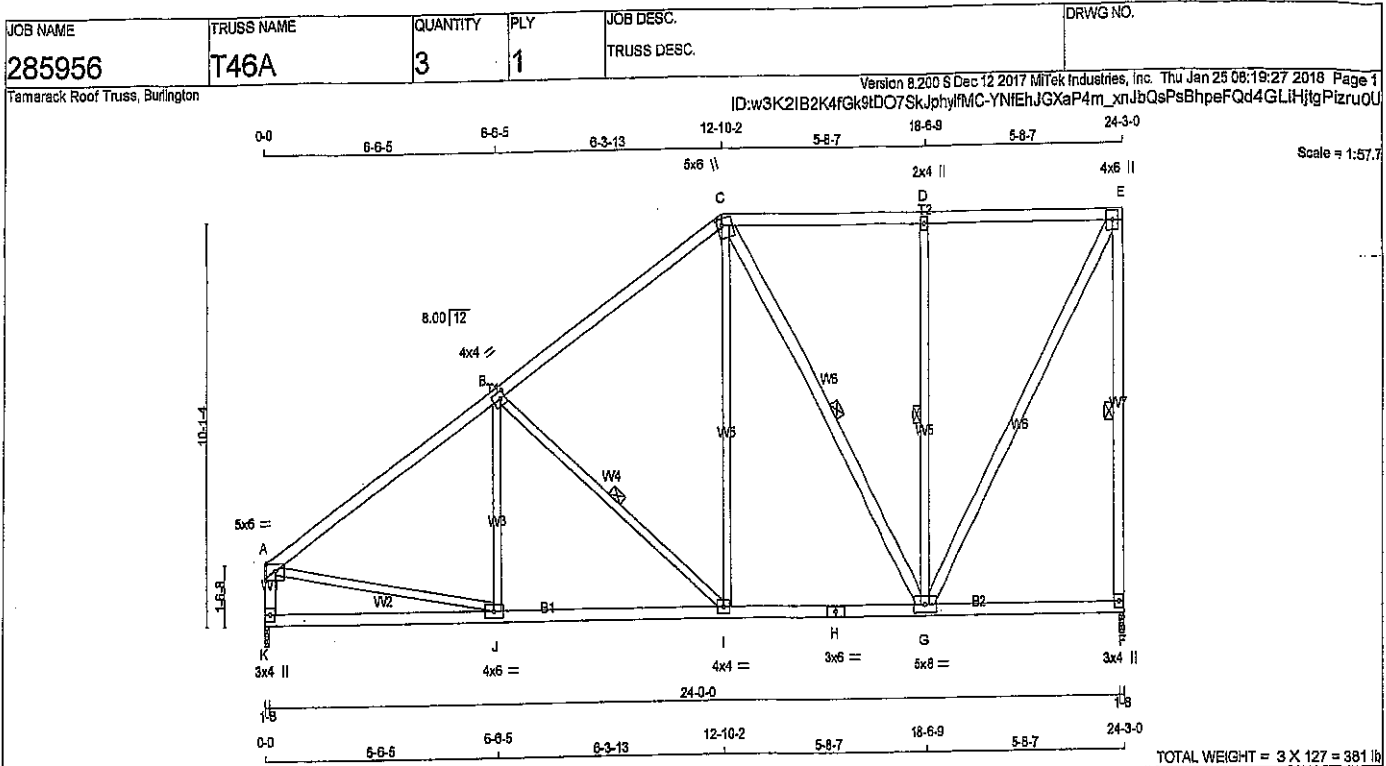
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (E) (INPUT = 0.90)
JSI METAL= 0.41 (H) (INPUT = 1.00)



DWG NO. TAM *5075* -18
STRUCTURAL
COMPONENT ONLY





LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

ALL WEBS

EXCEPT

CHORDS	SIZE	DRY	LUMBER	DESCR.
C - G	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	6.0	Edge	
B	TMVW-t	MT20	4.0	4.0	2.00	1.50
C	TTVW+m	MT20	5.0	6.0	2.00	1.50
D	TMVW-w	MT20	2.0	4.0		
E	TMVW+p	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMVWV-t	MT20	5.0	8.0		
H	BS-t	MT20	3.0	6.0		
I	BMVW-t	MT20	4.0	4.0		
J	BMVW-t	MT20	4.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
F	1381	0	1381	0	1-8	1-8
K	1381	0	1381	0	1-8	1-8

UNFACTORED REACTIONS							
JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1118	620 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
K	1118	620 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-F, B-I, C-G, D-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)		
FR-TO		FROM	TO		FR-TO				
A-B	-1481 / 0	-84.3	-84.3	0.52 (1)	4.77	J-B	-29 / 248	0.06 (3)	
B-C	-1050 / 0	-84.3	-84.3	0.48 (1)	5.50	B-I	-557 / 0	0.27 (1)	
C-D	-651 / 0	-84.3	-84.3	0.35 (1)	6.25	I-C	0 / 531	0.12 (2)	
D-E	-651 / 0	-84.3	-84.3	0.35 (1)	6.25	C-G	-393 / 0	0.28 (1)	
E-F	-1296 / 0	0.0	0.0	0.65 (1)	5.67	G-D	-595 / 0	0.42 (1)	
K-A	-1266 / 0	0.0	0.0	0.13 (1)	7.11	G-E	0 / 1288	0.21 (1)	
						A-J	0 / 1284	0.23 (1)	
K-J	0 / 0	-28.0	-28.0	0.30 (3)	10.00				
J-I	0 / 1262	-28.0	-28.0	0.44 (2)	10.00				
I-H	0 / 647	-28.0	-28.0	0.32 (2)	10.00				
H-G	0 / 647	-28.0	-28.0	0.32 (2)	10.00				
G-F	0 / 0	-28.0	-28.0	0.22 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	PSF
LL	25.8	PSF
BOT CH.	LL	PSF
LL	10.5	PSF
DL	7.0	PSF
TOTAL LOAD	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 069-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 9.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL)= L/999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL)= L/999 (0.13")

CSI: TC=0.65/1.00 (E-F:1), BC=0.44/1.00 (J:2), WB=0.42/1.00 (D-G:1), SS=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822
		2284 1656

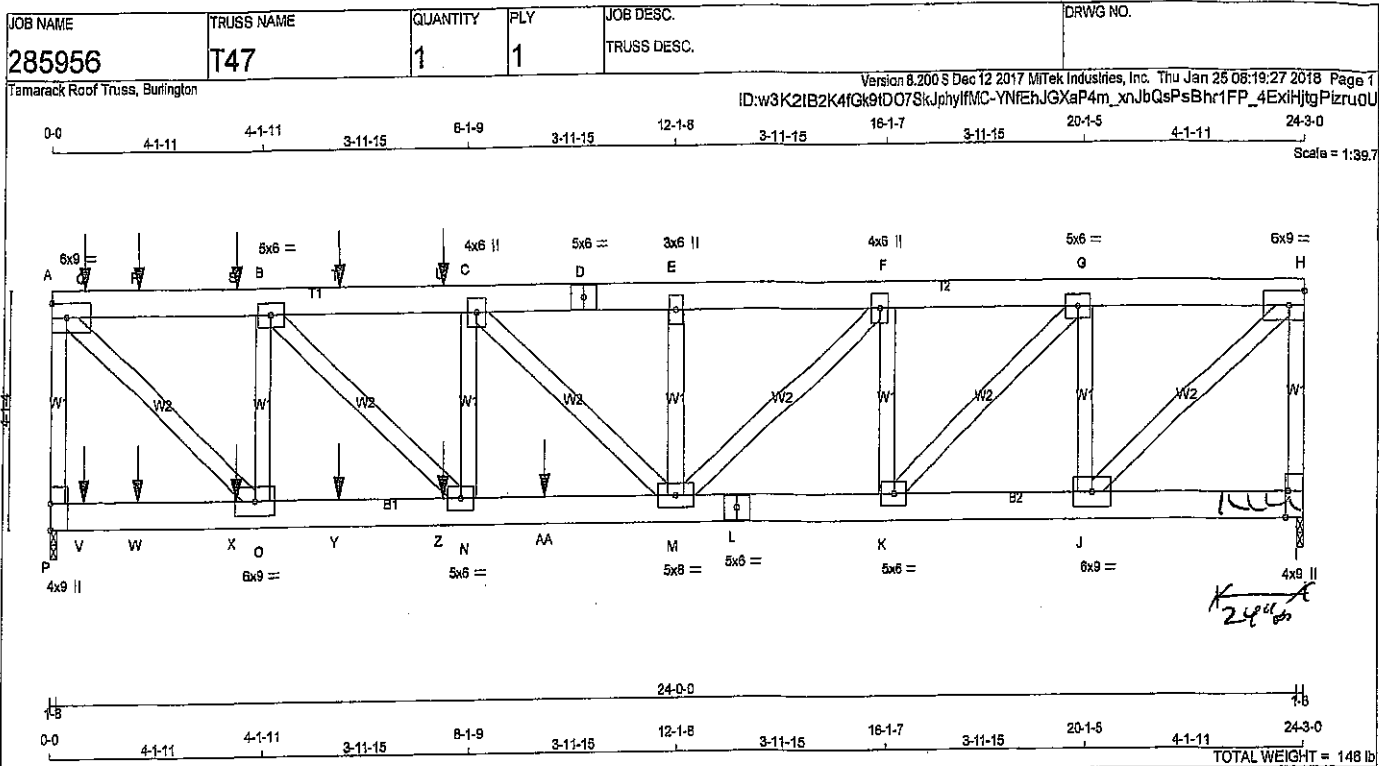
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (J) (INPUT = 0.90)
JSI METAL= 0.32 (J) (INPUT = 1.00)



DRWG NO. TAM 5985-18
STRUCTURAL
COMPONENT ONLY



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
P - A	2x4	DRY No.2	SPF
A - D	2x6	DRY 2100F 1.8E	SPF
D - H	2x6	DRY 2100F 1.8E	SPF
I - H	2x4	DRY No.2	SPF
P - L	2x6	DRY 2100F 1.8E	SPF
L - I	2x6	DRY 2100F 1.8E	SPF

ALL WEBS 2x4 DRY No.2
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-I	MT20	6.0	9.0		Edge
B	TMWW-I	MT20	5.0	5.0		
C	TMWW-I	MT20	4.0	8.0		
D	TS-I		5.0	6.0		
E	TMWW-I	MT20	3.0	6.0		
F	TMWW-I	MT20	4.0	6.0		
G	TMWW-I	MT20	5.0	8.0		
H	TMWW-I	MT20	6.0	9.0		Edge
I	BMV-I	MT20	4.0	9.0		Edge 0.50
J	BMWW-I	MT20	6.0	9.0		
K	BMWW-I	MT20	5.0	6.0		
L	BS-I	MT20	5.0	6.0		
M	BMWW-I	MT20	5.0	8.0		
N	BMWW-I	MT20	5.0	8.0		
O	BMWW-I	MT20	6.0	9.0		
P	BMV-I	MT20	4.0	9.0		Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 116.3 lbs FACTORED DOWN AT 7-12, 101.4 lbs FACTORED DOWN AT 1-7-12, 101.4 lbs FACTORED DOWN AT 3-7-12, AND 101.4 lbs FACTORED DOWN AT 5-7-12, AND 101.4 lbs FACTORED DOWN AT 7-12 ON TOP CHORD, AND 78.6 lbs FACTORED DOWN AT 7-12, 69.9 lbs FACTORED DOWN AT 1-7-12, 69.9 lbs FACTORED DOWN AT 3-7-12, 69.9 lbs FACTORED DOWN AT 5-7-12, AND 69.9 lbs FACTORED DOWN AT 7-12, AND 96.3 lbs FACTORED DOWN AT 9-7-0 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

SCAB JOINT # P WITH 2 X 6 #2
SPF 24" LONG, 1 SIDE(S) USING 2
ROW(S) 3/4" COMMON WIRE NAILS
AT 2" O/C STAGGERED (20 NAILS/SCAB)
REQUIRED FOR 1/2" BRG PROVISION.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
P	2555	0	2555	0	1-8	1-8
I	1854	0	1854	0	1-8	2-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
P	2123	1141 / 0	809 / 0	0 / 0	0 / 0	473 / 0	0 / 0
I	1532	835 / 0	360 / 0	0 / 0	0 / 0	338 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.15 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)		
FR-TO		FROM TO			FR-TO				
P-A	-2425 / 0	0.0	0.0	0.56 (1)	5.43	A-O	0 / 3221 0.57 (1)		
A-Q	-2417 / 0	-84.3	-84.3	0.12 (1)	6.18	O-B	-2010 / 0 0.34 (1)		
Q-R	-2417 / 0	-84.3	-84.3	0.12 (1)	6.16	B-N	0 / 2013 0.38 (1)		
R-S	-2417 / 0	-84.3	-84.3	0.12 (1)	6.18	N-C	-487 / 0 0.08 (1)		
S-T	-2417 / 0	-84.3	-84.3	0.12 (1)	6.18	C-M	0 / 88 0.02 (1)		
T-U	-3903 / 0	-84.3	-84.3	0.12 (1)	5.15	M-E	-329 / 0 0.06 (1)		
U-C	-3903 / 0	-84.3	-84.3	0.12 (1)	5.15	E-F	0 / 1152 0.20 (1)		
C-D	-3989 / 0	-84.3	-84.3	0.08 (1)	5.15	F-G	-1108 / 0 0.19 (1)		
D-E	-3969 / 0	-84.3	-84.3	0.08 (1)	5.15	G-K	0 / 1681 0.30 (1)		
E-F	-3989 / 0	-84.3	-84.3	0.08 (1)	5.16	K-G	-1513 / 0 0.28 (1)		
F-G	-3118 / 0	-84.3	-84.3	0.08 (1)	5.67	J-H	0 / 2500 0.44 (1)		
G-H	-1876 / 0	-84.3	-84.3	0.06 (1)	6.25				
H-I	-1802 / 0	0.0	0.0	0.41 (1)	6.18				

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	START
Q	7-12	-116	-118	-	FRONT	VERT	TOTAL
R	1-7-12	-101	-101	-	FRONT	VERT	TOTAL
S	3-7-12	-101	-101	-	FRONT	VERT	TOTAL
T	5-7-12	-101	-101	-	FRONT	VERT	TOTAL
U	7-12	-101	-101	-	FRONT	VERT	TOTAL
V	7-12	-45	-79	-	FRONT	VERT	TOTAL
W	1-7-12	-40	-70	-	FRONT	VERT	TOTAL
X	3-7-12	-40	-70	-	FRONT	VERT	TOTAL

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	START
Q	7-12	-116	-118	-	FRONT	VERT	TOTAL
R	1-7-12	-101	-101	-	FRONT	VERT	TOTAL
S	3-7-12	-101	-101	-	FRONT	VERT	TOTAL
T	5-7-12	-101	-101	-	FRONT	VERT	TOTAL
U	7-12	-101	-101	-	FRONT	VERT	TOTAL
V	7-12	-45	-79	-	FRONT	VERT	TOTAL
W	1-7-12	-40	-70	-	FRONT	VERT	TOTAL
X	3-7-12	-40	-70	-	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 45.1 PSF

SPACING = 24.0 IN. O.C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOFLIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/999 (0.13")
ALLOWABLE DEFL.(TL) = L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/999 (0.21")

CSI: TC=0.58/1.00 (A-P:1), BC=0.48/1.00 (M-N:1),
WB=0.57/1.00 (A-O:1), SS=0.46/1.00 (M-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

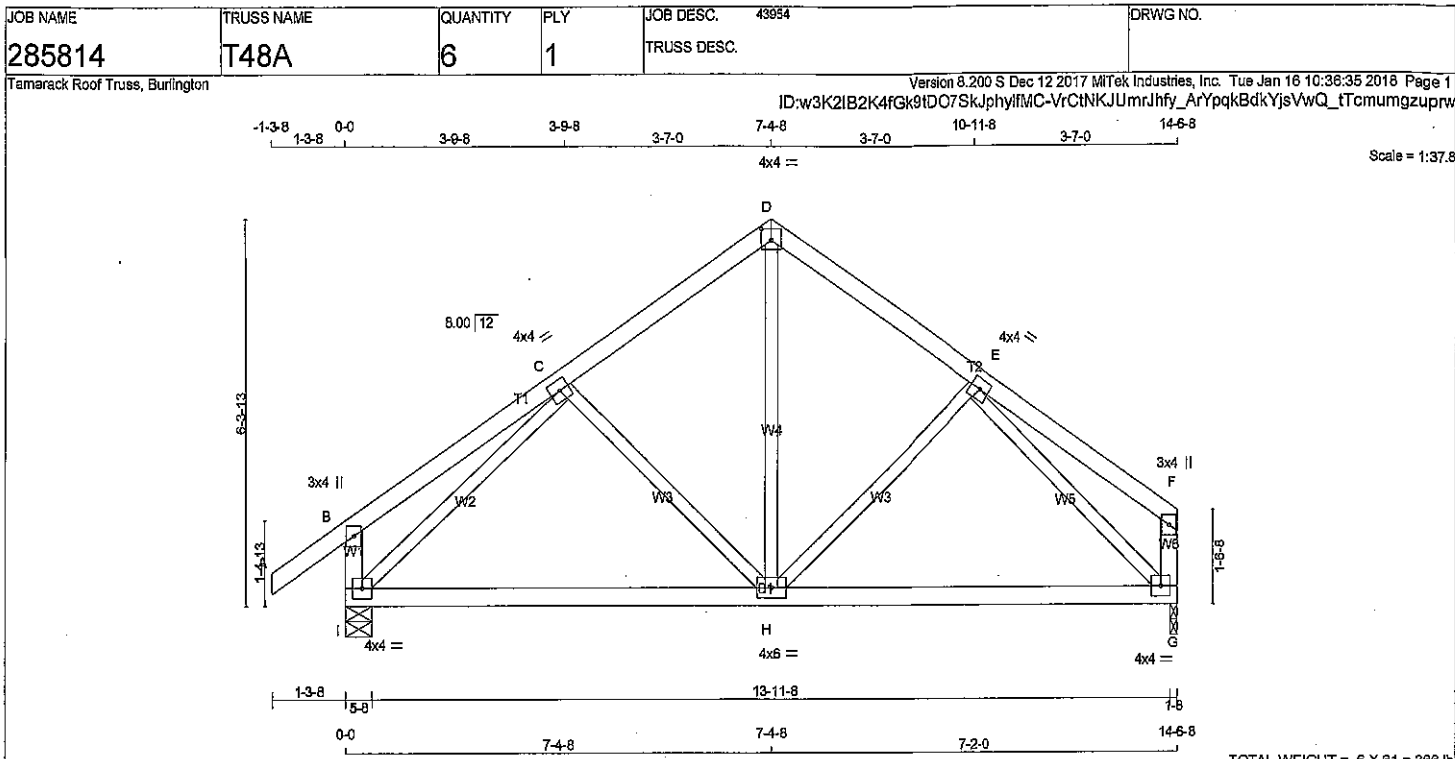
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1657 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (K) (INPUT = 0.90)
JSI METAL= 0.77 (L) (INPUT = 1.00)

DWG NO. TAM 5986-18
STRUCTURAL
COMPONENT ONLY



LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
C	TMVW-t	MT20	4.0	4.0	
D	TTW-p	MT20	4.0	4.0	2.25 2.00
E	TMVW-t	MT20	4.0	4.0	
F	TMV+p	MT20	3.0	4.0	
G	BMVW1-t	MT20	4.0	4.0	
H	BMVW1-t	MT20	4.0	6.0	
I	BMVW1-t	MT20	4.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
I	932	0	932	0	5-8	5-8
G	816	0	816	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
I	749	442 / 0	153 / 0	0 / 0	0 / 0	154 / 0	0 / 0
G	670	372 / 0	153 / 0	0 / 0	0 / 0	145 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	
FR-TO				FR-TO			
A-B	0 / 32	-84.3	-84.3 0.11 (1)	10.00	H-D	0 / 455	0.10 (1)
B-C	0 / 20	-84.3	-84.3 0.18 (1)	10.00	H-E	-130 / 53	0.06 (1)
C-D	-649 / 0	-84.3	-84.3 0.14 (1)	6.25	C-H	-164 / 42	0.07 (1)
D-E	-648 / 0	-84.3	-84.3 0.13 (1)	6.25	I-C	-891 / 0	0.37 (1)
E-F	0 / 20	-84.3	-84.3 0.17 (1)	10.00	E-G	-882 / 0	0.34 (1)
I-B	-235 / 0	0.0	0.0 0.02 (1)	7.81			
G-F	-111 / 0	0.0	0.0 0.01 (1)	7.81			
I-H	0 / 843	-28.0	-28.0 0.50 (2)	10.00			
H-G	0 / 818	-28.0	-28.0 0.50 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.8 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.48")
CALCULATED VERT. DEFL.(LL) = $L/899$ (0.09")
ALLOWABLE DEFL.(TL) = $L/360$ (0.48")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.15")

CSI: TC=0.16/1.00 (B-C:1), BC=0.50/1.00 (H-I:2), WB=0.37/1.00 (C-I:1), SI=0.17/1.00 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 622 2284 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

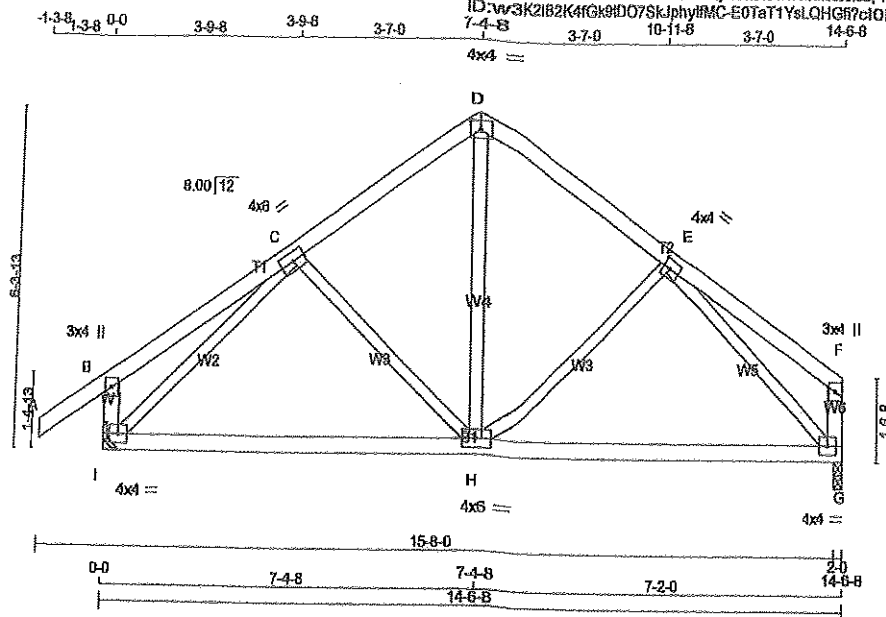
JSI GRIP= 0.77 (E) (INPUT = 0.90)
JSI METAL= 0.32 (C) (INPUT = 1.00)



DWG NO. TAM3983-18
STRUCTURAL
COMPONENT ONLY

JOB NAME 285745	TRUSS NAME T48AA	QUANTITY 3	PLY 1	JOB DESC. 43954	TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington						

Version 8.210 S May 18 2018 Mitek Industries, Inc. Tue Jun 19 15:28:07 2018 Page 1
 ID: w3K2iB2K4fGk8iDO7SkJphyIMC-E0TaT1YsLQHGR7cl0hARL0nHK8P1ndsYFtqDCz4hes
 7-4-8 3-7-0 10-11-8 3-7-0 14-6-8
 1-3-8 0-0 3-9-8 3-9-8 3-7-0



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4 DRY	No.2	SPF
D - F	2x4 DRY	No.2	SPF
I - B	2x4 DRY	No.2	SPF
G - F	2x4 DRY	No.2	SPF
I - G	2x4 DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	M120	3.0	4.0		
C	TMVW-1	M120	4.0	6.0		
D	TTW+p	M120	4.0	4.0	2.25	2.00
E	TMVW-1	M120	4.0	4.0	2.00	1.75
F	TMV+p	M120	3.0	4.0		
G	BMVW-1	M120	4.0	4.0		
H	BMVW-1	M120	4.0	6.0		
I	BMVW-1	M120	4.0	4.0		

DESIGNATIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
G	816	0	816	0	2-0			
I	932	0	932	0	0			MECHANICAL

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT I.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX	MIN	COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL	
G	870	372 / 0	153 / 0	0 / 0	0 / 0	145 / 0	0 / 0	
I	749	442 / 0	153 / 0	0 / 0	0 / 0	154 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TD		FROM	TO	FR-TD			
A-B	0 / 32	-84.3	-84.3 0.11 (1)	10.00	H-D	0 / 456	0.10 (1)
B-C	0 / 20	-84.3	-84.3 0.10 (1)	10.00	H-E	-130 / 53	0.06 (1)
C-D	-849 / 0	-84.3	-84.3 0.14 (1)	8.25	C-H	-184 / 41	0.07 (1)
D-E	-848 / 0	-84.3	-84.3 0.13 (1)	8.25	E-C	-891 / 0	0.37 (1)
E-F	0 / 19	-84.3	-84.3 0.17 (1)	10.00	E-G	-882 / 0	0.34 (1)
I-B	-236 / 0	0.0	0.0 0.02 (1)	7.81			
G-F	-111 / 0	0.0	0.0 0.01 (1)	7.81			
I-H	0 / 643	-28.0	-28.0 0.50 (2)	10.00			
H-G	0 / 618	-28.0	-28.0 0.50 (2)	10.00			

TOTAL WEIGHT = 3 X G1 = 183 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	25.6	PSF
DL	=	3.0	PSF
BOT CH. LL	=	10.5	PSF
DL	=	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. CG

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 9.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.48")
CALCULATED VERT. DEFL. (LL) = 1 / 699 (0.09")
ALLOWABLE DEFL. (TL) = L/360 (0.48")
CALCULATED VERT. DEFL. (TL) = 1 / 699 (0.15")

CS1: TC=0.18/1.00 (B-C:1), BC=0.50/1.00 (H:2), WB=0.37/1.00 (C:1), SS=0.17/1.00 (H:3)

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PL)
MAX MIN	MAX MIN	MAX MIN
M120	619 354 1867 622	2284 1056

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.89 (E) (INPUT= 0.90)
JSI METAL= 0.31 (E) (INPUT= 1.00)

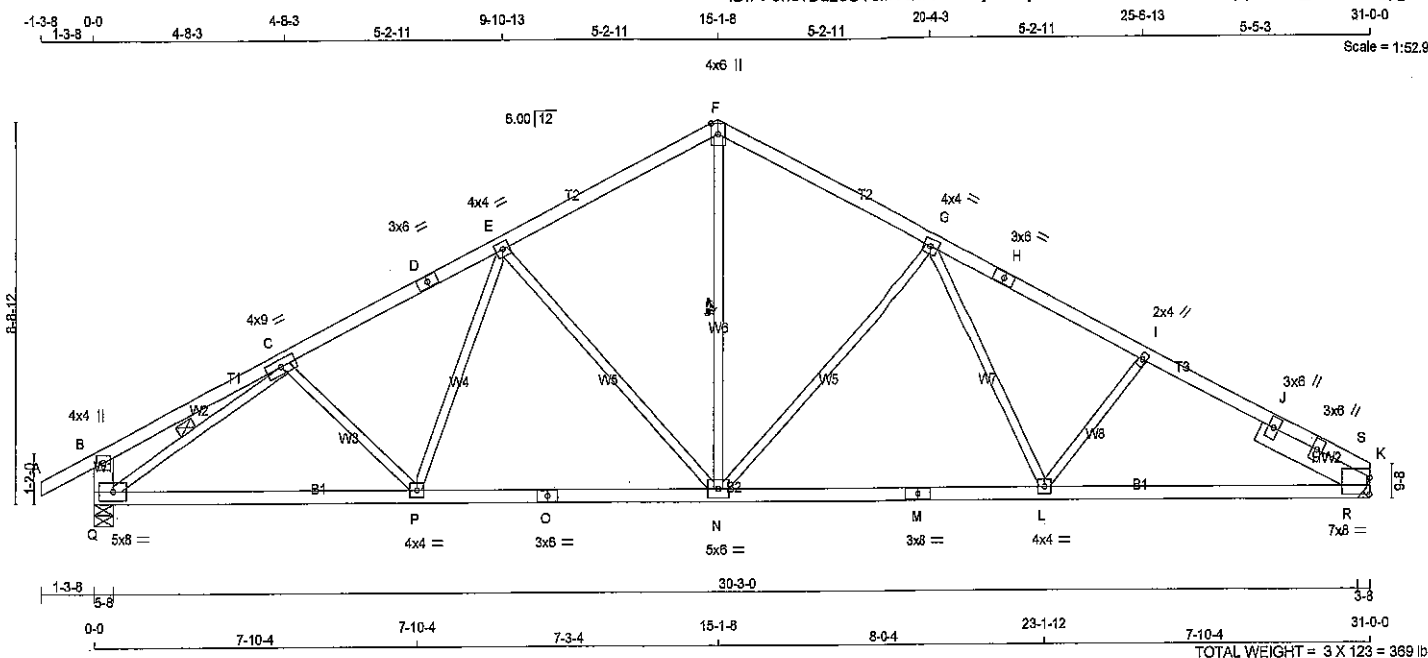


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
285861	T71X	3	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 3 X 123 = 369 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
Q - B	2x6	DRY	No.2
Q - O	2x4	DRY	No.2
O - M	2x4	DRY	No.2
M - K	2x4	DRY	No.2

REINFORCING MEMBERS			
HW2	2x6	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	4.0	4.0
C	TMWW-t	MT20	4.0	8.0
D	TS-t	MT20	3.0	6.0
E	TMWW-t	MT20	4.0	4.0
F	TTW+p	MT20	4.0	6.0
G	TMWW-t	MT20	4.0	4.0
H	TS-t	MT20	3.0	6.0
I	TMW+w	MT20	2.0	4.0
K	TMBSR-t	MT20	7.0	6.0
K	RT-t	MT20	3.0	6.0
K	RT-t	MT20	3.0	6.0
L	BMWW-t	MT20	4.0	4.0
M	BS-t	MT20	3.0	8.0
N	BMWW-t	MT20	5.0	6.0
O	BS-t	MT20	3.0	6.0
P	BMWW-t	MT20	4.0	4.0
Q	BMWW-t	MT20	5.0	8.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	VERT	GROSS	REACT	DOWN	UP	BRG	IN-SX	BRG	IN-SX
Q	1854	0	1854	0	0	5-8	5-8		
K	1740	0	1740	0	0				

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN COMPONENT REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
Q	1506	863 / 0	326 / 0	0 / 0	0 / 0	316 / 0	0 / 0		
K	1429	793 / 0	326 / 0	0 / 0	0 / 0	310 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-Q. *F.N.E.*

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	CS (LC)	MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	CS (LC)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 26	-84.3	-84.3	0.11 (1)	10.00	C-P	-33 / 148
B-C	0 / 16	-84.3	-84.3	0.23 (1)	10.00	P-E	0 / 357
C-D	-2396 / 0	-84.3	-84.3	0.35 (1)	4.15	E-N	-680 / 0
D-E	-2396 / 0	-84.3	-84.3	0.35 (1)	4.15	N-F	0 / 1274
E-F	-1828 / 0	-84.3	-84.3	0.32 (1)	4.66	G-N	-782 / 0
F-G	-1831 / 0	-84.3	-84.3	0.34 (1)	4.62	G-L	0 / 455
G-H	-2552 / 0	-84.3	-84.3	0.40 (1)	3.98	L-I	-205 / 32
H-I	-2552 / 0	-84.3	-84.3	0.40 (1)	3.98	Q-C	-2835 / 0
I-J	-2053 / 0	-84.3	-84.3	0.39 (1)	4.35	R-S	0 / 1452
J-S	-2053 / 0	-84.3	-84.3	0.39 (1)	4.35	R-J	-1518 / 0
S-K	-2410 / 0	-84.3	-84.3	0.14 (1)	4.33		
Q-B	-271 / 0	0.0	0.0	0.02 (1)	7.81		

Q-P	0 / 2172	-28.0	-28.0	0.63 (2)	10.00
P-O	0 / 2058	-28.0	-28.0	0.62 (2)	10.00
O-N	0 / 2058	-28.0	-28.0	0.62 (2)	10.00
N-M	0 / 2128	-28.0	-28.0	0.56 (2)	10.00
M-L	0 / 2126	-28.0	-28.0	0.56 (2)	10.00
L-R	0 / 2429	-28.0	-28.0	0.61 (1)	10.00
R-K	0 / 1301	-28.0	-28.0	0.47 (2)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD	=	46.1	PSF	

SPACING = 24.0 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/999 (0.19")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/999 (0.33")

CSI: TC=0.40 (G-I:1), BC=0.63 (P-Q:2), WB=0.86 (G-N:1), SSI=0.35 (K-R:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.74 (M) (INPUT = 1.00)

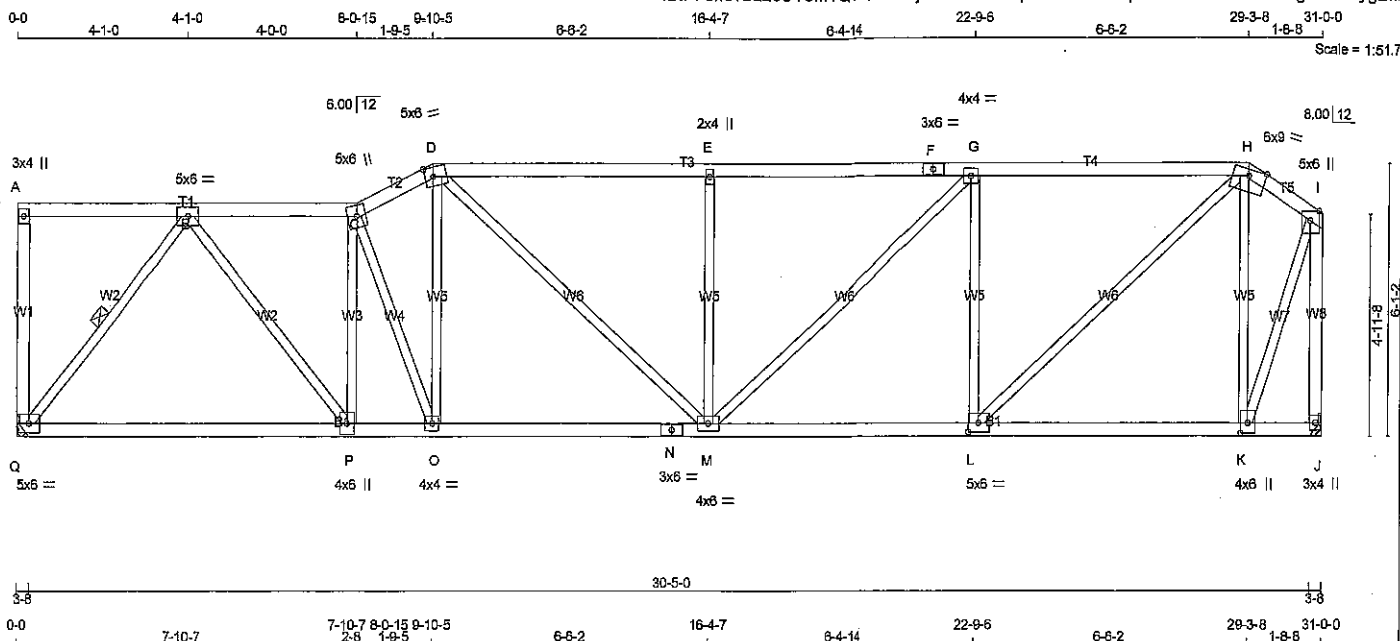


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
285861	T73S	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:PFehc1Ba2cOYemTQTAYm6Oy1tEX-BEaTeqYPmKJ9SrDwpmZRo8GvvRYIJW2ogX0GY6yg2le



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
Q - A	2x4	DRY	No.2
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - I	2x4	DRY	No.2
J - I	2x4	DRY	No.2
Q - N	2x4	DRY	No.2
N - J	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	5.0	6.0		
C	TTWW+m	MT20	5.0	6.0		
D	TTWW-m	MT20	5.0	6.0	2.50	2.25
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW-m	MT20	6.0	9.0	Edge	
I	TMVV+p	MT20	5.0	6.0	Edge	
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	4.0	6.0	2.75	2.00
L	BMWW-t	MT20	5.0	6.0	2.50	2.75
M	BMWW-t	MT20	4.0	6.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	4.0	6.0		
Q	BMVV-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT		REQRD	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	IN-SX	BRG	IN-SX
Q	1740	0	1740	0	0				
J	1740	0	1740	0	0				

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1429	793 / 0	326 / 0	0 / 0	0 / 0	310 / 0	0 / 0
J	1429	793 / 0	326 / 0	0 / 0	0 / 0	310 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.74 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF B-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM	TO		FR-TO			
Q-A	-131 / 0	0.0	0.0	0.05 (1)	7.81	P-C	-882 / 0	0.35 (1)
A-B	0 / 0	-84.3	-84.3	0.23 (1)	10.00	C-O	-359 / 0	0.16 (1)
B-C	-2131 / 0	-84.3	-84.3	0.30 (1)	4.35	O-D	0 / 399	0.09 (2)
C-D	-2280 / 0	-84.3	-84.3	0.10 (1)	4.48	K-H	-1178 / 0	0.69 (1)
D-E	-2390 / 0	-84.3	-84.3	0.69 (1)	3.75	K-I	0 / 1430	0.32 (1)
E-F	-2391 / 0	-84.3	-84.3	0.69 (1)	3.74	L-H	0 / 1854	0.42 (1)
F-G	-2391 / 0	-84.3	-84.3	0.69 (1)	3.74	D-M	0 / 485	0.11 (1)
G-H	-1855 / 0	-84.3	-84.3	0.62 (1)	4.19	L-G	-1051 / 0	0.61 (1)
H-I	-828 / 0	-84.3	-84.3	0.05 (1)	6.25	M-E	-592 / 0	0.35 (1)
I-J	-1755 / 0	0.0	0.0	0.77 (1)	6.29	M-G	0 / 682	0.15 (1)
						Q-B	-1972 / 0	0.55 (1)
Q-P	0 / 1260	-28.0	-28.0	0.54 (2)	10.00	B-P	0 / 1402	0.32 (1)
P-O	0 / 2150	-28.0	-28.0	0.66 (2)	10.00			
Q-N	0 / 2028	-28.0	-28.0	0.48 (2)	10.00			
N-M	0 / 2028	-28.0	-28.0	0.48 (2)	10.00			
M-L	0 / 1885	-28.0	-28.0	0.46 (2)	10.00			
L-K	0 / 500	-28.0	-28.0	0.29 (2)	10.00			
K-J	0 / 0	-28.0	-28.0	0.18 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL)= L/999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL)= L/999 (0.30")

CSI: TC=0.77 (I-J), BC=0.66 (O-P), WB=0.69 (H-K), SI=0.28 (G-H):

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

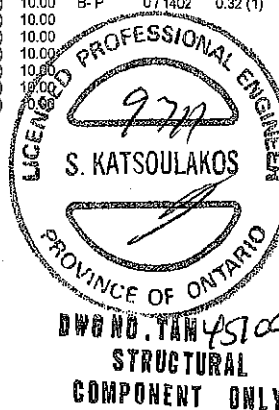
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1657 822 2284 1656

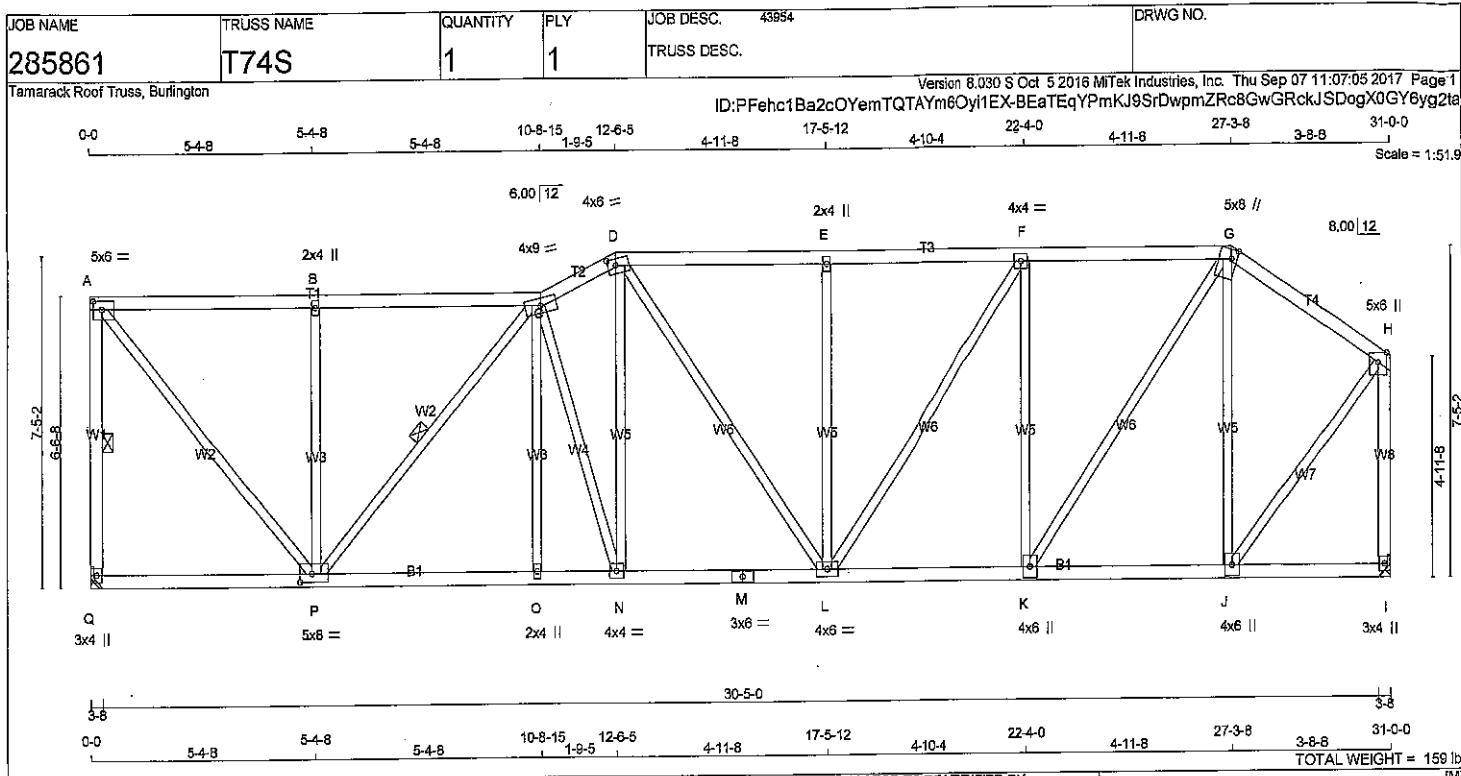
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.60 (D) (INPUT = 0.90)
JSI METAL= 0.55 (N) (INPUT = 1.00)



STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
Q - A	2x4	DRY No.2	SPF
A - C	2x4	DRY No.2	SPF
C - D	2x4	DRY No.2	SPF
D - G	2x4	DRY No.2	SPF
G - H	2x4	DRY No.2	SPF
H - I	2x4	DRY No.2	SPF
I - M	2x4	DRY No.2	SPF
M - J	2x4	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	8.0	2.25	2.50
B	TMVW-w	MT20	2.0	4.0		
C	TTWWW-m	MT20	4.0	9.0		
D	TTWW-m	MT20	4.0	6.0	1.75	2.25
E	TMVW-w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	4.0		
G	TTWWW+m	MT20	5.0	8.0	2.50	1.50
H	TMVW+p	MT20	5.0	6.0	Edge	
I	BMV1+p	MT20	3.0	4.0		
J	BMVW-t	MT20	4.0	6.0		
K	BMVW-t	MT20	4.0	6.0		
L	BMVW-t	MT20	4.0	6.0		
M	BS-t	MT20	3.0	6.0		
N	BMVW-t	MT20	4.0	4.0		
O	BMVW-w	MT20	2.0	4.0		
P	BMVW-t	MT20	5.0	8.0	2.25	3.50
Q	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
Q	1740	0	1740	0	0	HANGER BY OTHERS
I	1740	0	1740	0	0	MIN. SEAT SIZE: 3-8 HANGER BY OTHERS

UNFACTORED REACTIONS							
	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1429	793/0	326/0	0/0	0/0	310/0	0/0
I	1429	793/0	326/0	0/0	0/0	310/0	0/0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.53 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF A-Q, C-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS			
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO				
Q-A	-1679/0	0.0 0.0 0.33 (1)	5.13	A-P	0/1991	0.45 (1)
A-B	-1304/0	-84.3 -84.3 0.45 (1)	5.06	P-B	-559/0	0.39 (1)
B-C	-1304/0	-84.3 -84.3 0.45 (1)	5.06	P-C	-1011/0	0.45 (1)
C-D	-2035/0	-84.3 -84.3 0.09 (1)	4.89	O-C	0/148	0.03 (3)
D-E	-1892/0	-84.3 -84.3 0.33 (1)	4.55	C-N	-484/0	0.37 (1)
E-F	-1892/0	-84.3 -84.3 0.34 (1)	4.53	N-D	0/570	0.13 (1)
F-G	-1556/0	-84.3 -84.3 0.32 (1)	4.93	J-G	-858/0	0.85 (1)
G-H	-986/0	-84.3 -84.3 0.22 (1)	6.01	J-H	0/1290	0.29 (1)
H-I	-1702/0	0.0 0.0 0.75 (1)	6.37	K-G	0/1312	0.30 (1)
				D-L	0/111	0.03 (1)
Q-P	0/0	-28.0 -28.0 0.21 (3)	10.00	K-F	-943/0	0.93 (1)
P-O	0/1991	-28.0 -28.0 0.44 (2)	10.00	L-E	-448/0	0.44 (1)
O-N	0/1990	-28.0 -28.0 0.37 (1)	10.00	L-F	0/598	0.13 (1)
N-M	0/1829	-28.0 -28.0 0.36 (1)	10.00			
M-L	0/1829	-28.0 -28.0 0.36 (1)	10.00			
L-K	0/1558	-28.0 -28.0 0.32 (1)	10.00			
K-J	0/807	-28.0 -28.0 0.23 (2)	10.00			
J-I	0/0	-28.0 -28.0 0.13 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 8 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/999 (0.19")

CSI: TC=0.75 (H-I:1), BC=0.44 (O-P:2), WB=0.93 (F-K:1), SS=0.22 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

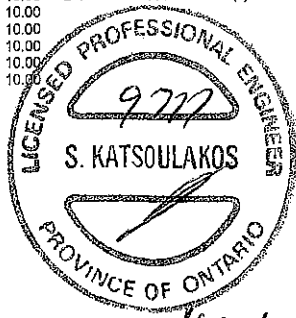
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 518 354 1687 822 2284 1658

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

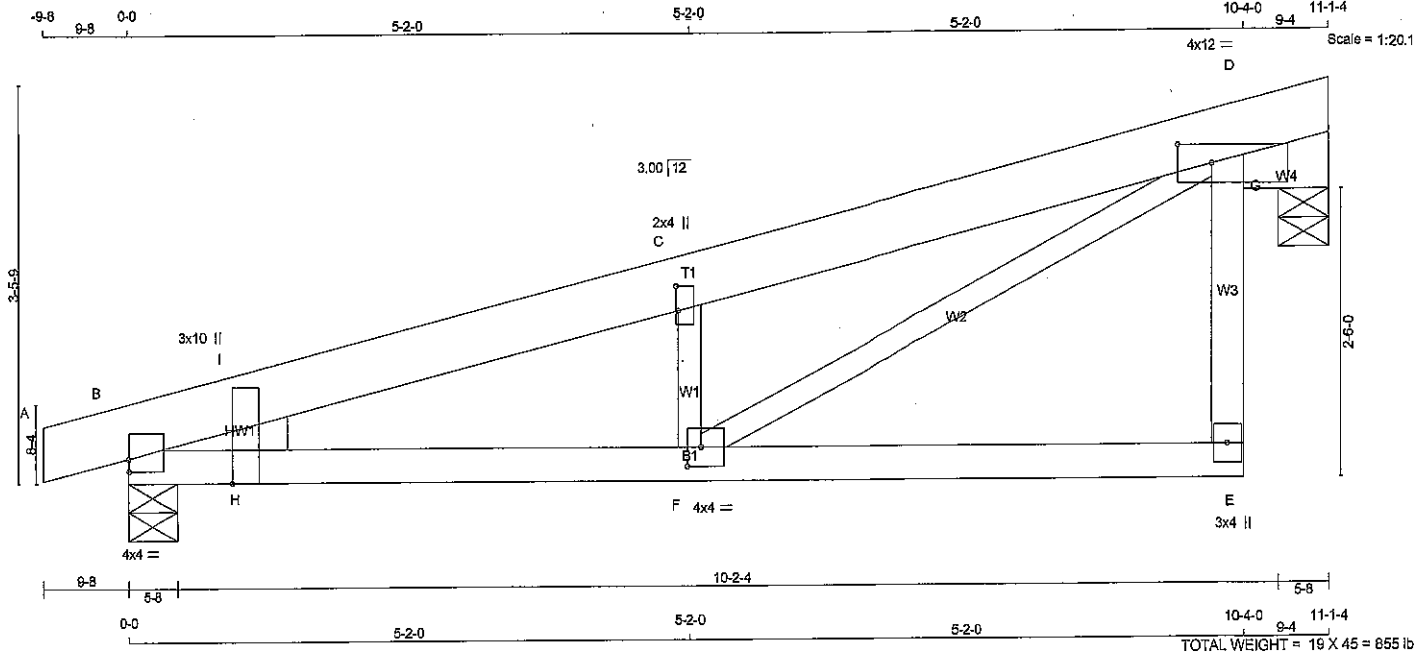
JSI GRIP= 0.89 (P) (INPUT = 0.90)
JSI METAL= 0.62 (C) (INPUT = 1.00)



DWG NO. TAM 45701-17
STRUCTURAL
COMPONENT ONLY

JOB NAME 285861	TRUSS NAME T90TCX	QUANTITY 19	PLY 1	JOB DESC. 43854	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.030 S Oct 5 2016 Mitek Industries, Inc. Thu Sep 07 11:07:05 2017 Page 1			

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LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x8	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
B TMB1-I	MT20	4.0	4.0	1.25
B WP-H	MT20	3.0	10.0	Edge 11.50
C TMW+H	MT20	2.0	4.0	2.50 0.25
D TMVWW1-p	MT20	4.0	12.0	2.00 3.75
E BMV+p	MT20	3.0	4.0	
F BMWW-I	MT20	4.0	4.0	2.00 1.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG	HEEL WEDGE
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	
J(D)	551	0	551	0	5-8	5-8
B	680	0	680	0	5-8	5-8

UNFACTORED REACTIONS		1ST LCASE	MAX/MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE	PERM.LIVE
J(D)	452	251 / 0	103 / 0	0 / 0
B	548	321 / 0	114 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J(D), B

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING		TOTAL LOAD CASES: (4)			
CHORDS		MEMB.		WEBS	
MAX. FACTORED	FORCE (LBS)	MAX. FACTORED	FORCE (LBS)	MAX. FACTORED	FORCE (LBS)
FR-TO		FR-TO		FR-TO	
A-B	-4 / 0	A-B	-420 / 0	A-B	-420 / 0
B-I	-1258 / 0	B-I	-1157 / 0	B-I	-1157 / 0
I-C	-1158 / 0	I-C	-785 / 0	I-C	-785 / 0
C-D	-1152 / 0	C-D	0 / 106	C-D	0 / 106
E-G	0 / 106	E-G	0 / 106	E-G	0 / 106
G-D	0 / 106	G-D	0 / 106	G-D	0 / 106
B-H	0 / 1128	B-H	0 / 1128	B-H	0 / 1128
H-F	0 / 1128	H-F	0 / 1128	H-F	0 / 1128
F-E	0 / 103	F-E	0 / 103	F-E	0 / 103

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/999 (0.06")

CSI: TC=0.34 (E-G:1), BC=0.25 (F-H:2), WB=0.26 (D-F:1), SSI=0.15 (D-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1887 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (F) (INPUT = 0.90)
JSI METAL= 0.40 (B) (INPUT = 1.00)

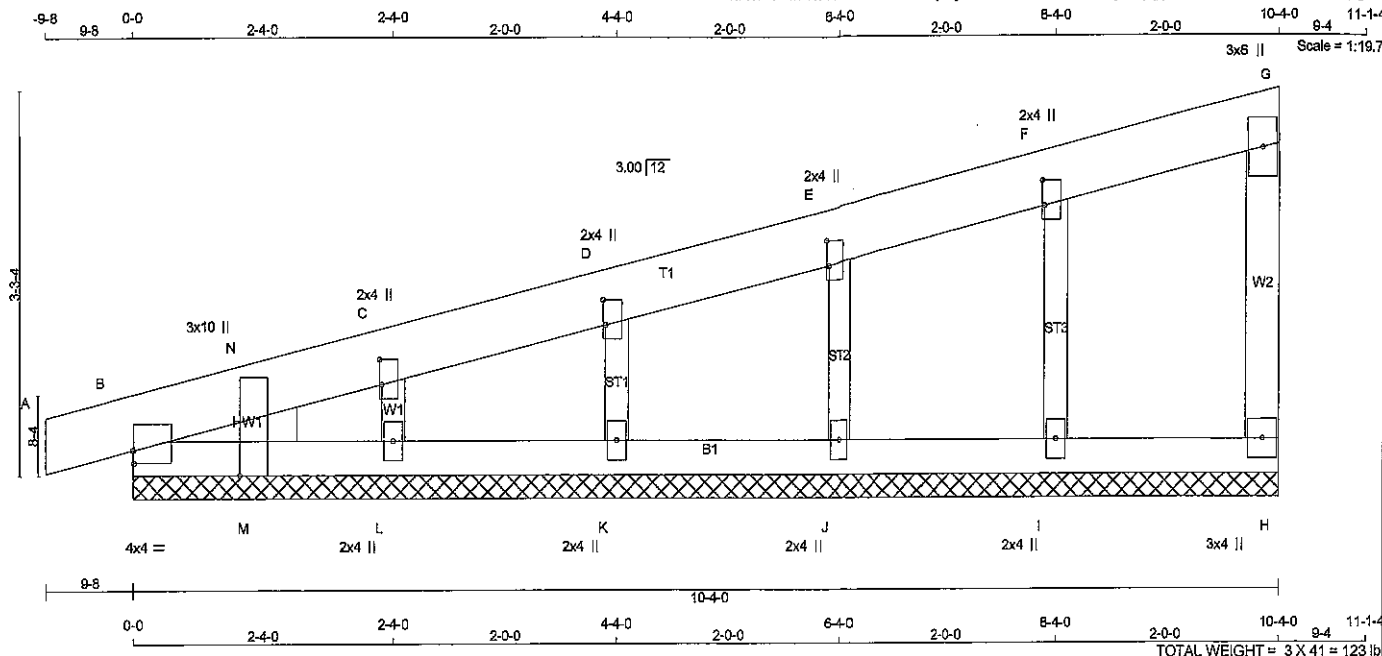


DWG NO. TAM 45102-17
STRUCTURAL
COMPONENT ONLY

JOB NAME 285861	TRUSS NAME G90	QUANTITY 3	PLY 1	JOB DESC. 43954 TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:w3K2IB2K4IGk8ID07SkJphylfMC-07PJw3PVXwje8tpgytsgpJCH?ycEibA8JrAfFyg2t



LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - G	2x8	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
B - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				
GABLE STUDS SPACED AT 2'-0" O.C.				

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TM61-H	MT20	4.0	4.0	1.25	
B	WPH	MT20	3.0	10.0	Edge	11.50
C, D, E, F						
C	TMW+w	MT20	2.0	4.0	2.50	0.25
G	TMV+p	MT20	3.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I, J, K, L						
I	BMW1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

HEEL
WEDGE
2x4 L
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	-410	-84.3 -84.3	0.02 (1)	I-F	-188 / 0	0.03 (1)	
B-N	-46 / 0	-84.3 -84.3	0.02 (1)	J-E	-188 / 0	0.03 (1)	
N-C	-14 / 3	-84.3 -84.3	0.03 (1)	K-D	-158 / 0	0.02 (1)	
C-D	-18 / 0	-84.3 -84.3	0.03 (1)	L-C	-201 / 0	0.03 (1)	
D-E	-9 / 0	-84.3 -84.3	0.02 (1)	M-N	0 / 41	0.00 (1)	
E-F	-4 / 0	-84.3 -84.3	0.02 (1)				
F-G	-4 / 0	-84.3 -84.3	0.02 (1)				
H-G	-72 / 0	0.0 0.0	0.01 (1)				
B-M	0 / 28	-28.0 -28.0	0.02 (1)				
M-L	0 / 28	-28.0 -28.0	0.03 (2)				
L-K	0 / 18	-28.0 -28.0	0.02 (2)				
K-J	0 / 9	-28.0 -28.0	0.02 (2)				
J-I	0 / 4	-28.0 -28.0	0.03 (2)				
I-H	0 / 0	-28.0 -28.0	0.03 (3)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. GIC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.03 (C-D:1), BC=0.03 (I-J:2), WB=0.03 (F-I:1), SSI=0.07 (C-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

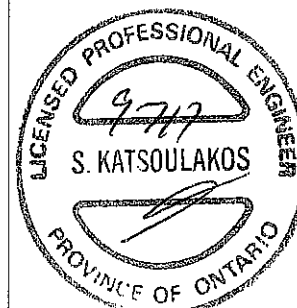
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (C) (INPUT = 0.90)
JSI METAL= 0.04 (L) (INPUT = 1.00)



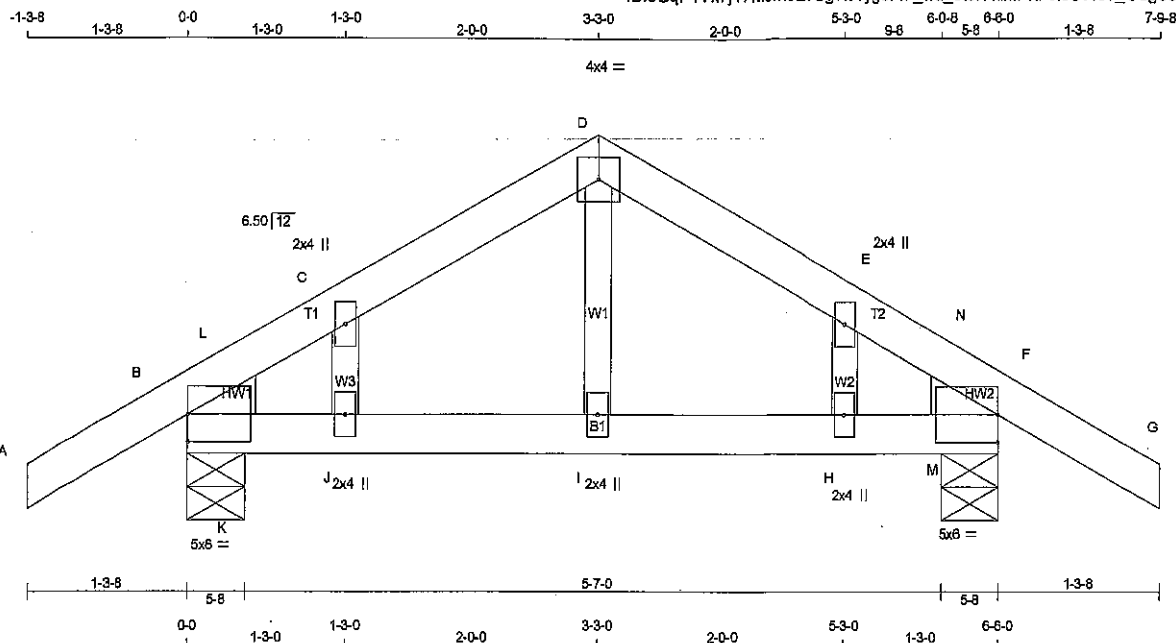
DRG NO. TAM 45137-17
STRUCTURAL
COMPONENT ONLY

JOB NAME 285862	TRUSS NAME G91	QUANTITY 1	PLY 1	JOB DESC. 43954	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1:17.3

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
B - F	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMBH1-I	MT20	5.0	6.0	2.50
C	TMW+w	MT20	2.0	4.0	
D	TTW-p	MT20	4.0	4.0	
E	TMW+w	MT20	2.0	4.0	
F	TMBH1-I	MT20	5.0	6.0	2.50
H, I, J					
H	BMW+w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS								
FACTORED GROSS REACTION			MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REORD BRG	HEEL
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	WEDGE
B	480	0	480	0	0	5-8	5-8	2x4 L
F	480	0	480	0	0	5-8	5-8	2x4 R

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	377	238 / 0	88 / 0	0 / 0	0 / 0	73 / 0	0 / 0
F	377	238 / 0	65 / 0	0 / 0	0 / 0	73 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC) (1)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC) (1)	
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 17	-84.3	-84.3	I-D	0 / 151	0.03	(2)
B-L	-525 / 0	-84.3	-84.3	H-E	-81 / 26	0.01	(1)
L-C	-322 / 0	-84.3	-84.3	J-C	-81 / 26	0.01	(1)
C-D	-298 / 0	-84.3	-84.3	K-L	0 / 225	0.00	(1)
D-E	-298 / 0	-84.3	-84.3	M-N	0 / 227	0.00	(1)
E-N	-322 / 0	-84.3	-84.3				
N-F	-527 / 0	-84.3	-84.3				
F-G	0 / 17	-84.3	-84.3				
B-K	0 / 267	-28.0	-28.0				
K-J	0 / 287	-28.0	-28.0				
J-I	0 / 281	-28.0	-28.0				
I-H	0 / 261	-28.0	-28.0				
H-M	0 / 267	-28.0	-28.0				
M-F	0 / 267	-28.0	-28.0				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/380 (0.22")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/380 (0.22")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.11/1.00 (F-N:1), BC=0.11/1.00 (H-M:1), WB=0.03/1.00 (D-I:2), SS=0.10/1.00 (F-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.66 (F) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)



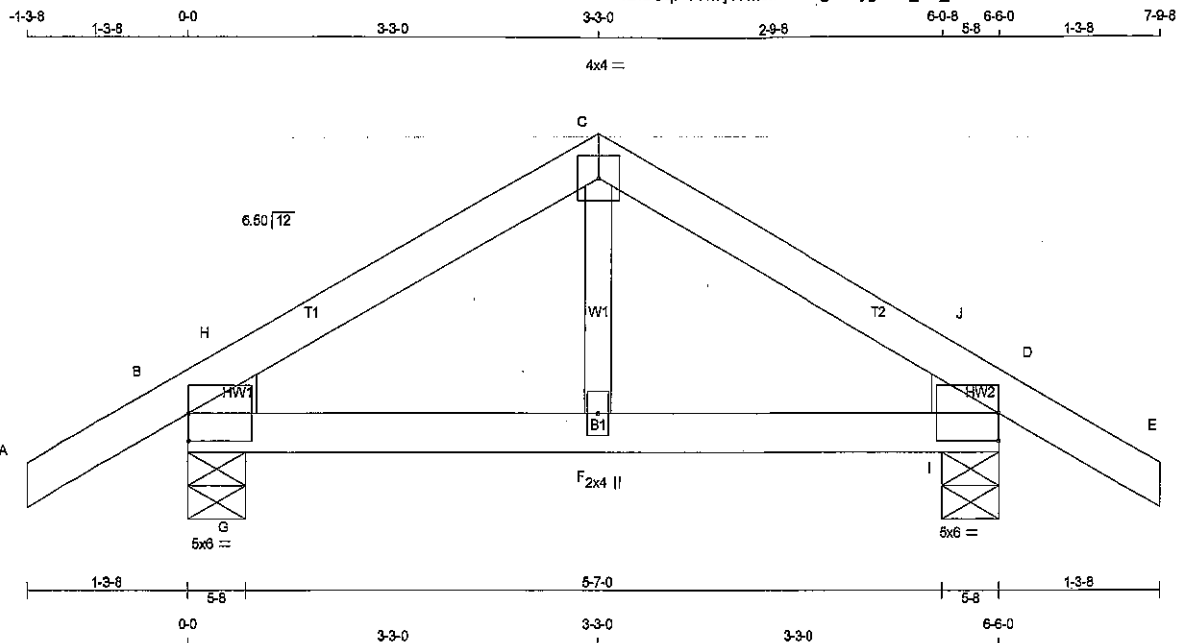
DRWG NO. TAM 209-18
STRUCTURAL
COMPONENT ONLY

JOB NAME 285862	TRUSS NAME T91	QUANTITY 3	PLY 1	JOB DESC. 43954	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1:17.3

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
B - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD	HEEL
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	IN-SX	WEDGE
B	480	0	480	0	5-8	2x4 L
D	480	0	480	0	5-8	2x4 R

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	377	238/0	68/0	0/0	0/0	73/0	0/0
D	377	238/0	68/0	0/0	0/0	73/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	MAX. (PLF)	MEMB.	FORCE (LBS)	MAX. (PLF)	CS (LC)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0/17	-84.3	-84.3 0.11 (1)	F-C	0/165	0.04 (2)	
B-H	-411/0	-84.3	-84.3 0.05 (1)	G-H	-34/113	0.00 (1)	
H-C	-355/0	-84.3	-84.3 0.08 (1)	I-J	-33/114	0.00 (1)	
C-J	-355/0	-84.3	-84.3 0.08 (1)				
J-D	-412/0	-84.3	-84.3 0.05 (1)				
D-E	0/17	-84.3	-84.3 0.11 (1)				
B-G	0/305	-28.0	-28.0 0.06 (1)				
G-F	0/305	-28.0	-28.0 0.10 (1)				
F-I	0/305	-28.0	-28.0 0.10 (1)				
I-D	0/305	-28.0	-28.0 0.08 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.22")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.00")
ALLOWABLE DEFL. (TL) = $L/360$ (0.22")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.01")

CSI: TC=0.11/1.00 (A-B:1), BC=0.10/1.00 (F-G:1), WB=0.04/1.00 (C-F:2), SS=0.09/1.00 (C-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1567 822 2284 1556

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (D) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)



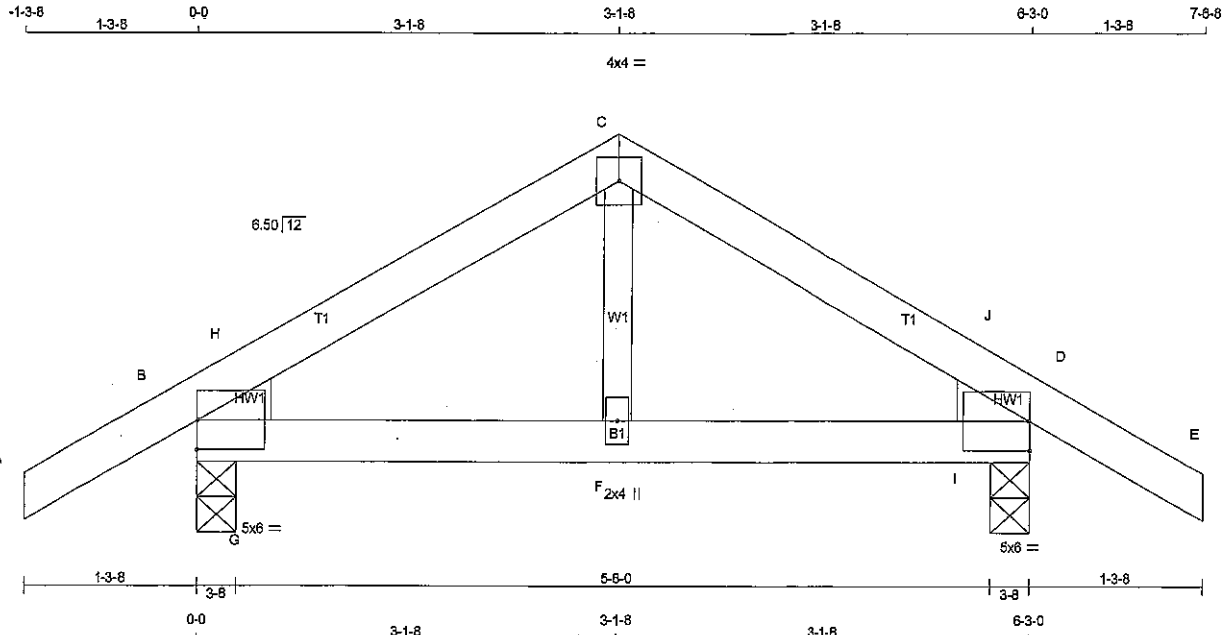
DWG NO. TAM 21210-18
STRUCTURAL
COMPONENT ONLY

JOB NAME 285861	TRUSS NAME T92	QUANTITY 4	PLY 1	JOB DESC. 43954 TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1:16.3

LUMBER
N. L. G. A. RULES
CHORDS SIZE

A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-I	MT20	5.0	6.0		
C	TTW-p	MT20	4.0	4.0		
D	TMBH1-I	MT20	5.0	6.0		
F	BMW-w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG	HEEL WEDGE
B	465	465	0	3-8	2x4 L
D	465	465	0	3-8	2x4 R

UNFACTORED REACTIONS

JT	1ST LCASE	MAX / MIN. COMPONENT REACTIONS	DEAD	SOIL
B	COMBINED	230 / 0	71 / 0	0 / 0
D	COMBINED	230 / 0	71 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 9.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX	MAX. UNBRAC LENGTH	MEMB.	WEBS MAX. FACTORED FORCE (LBS)	MAX	MAX. UNBRAC LENGTH
FR-TO		FROM	TO			FR-TO		FROM	TO
A-B	0 / 17	-84.3	-84.3	0.11 (1)	10.00	F-C	0 / 157	0.04 (2)	
B-H	-397 / 0	-84.3	-84.3	0.04 (1)	6.25	G-H	-28 / 107	0.00 (1)	
H-C	-338 / 0	-84.3	-84.3	0.07 (1)	6.25	I-J	-28 / 107	0.00 (1)	
C-J	-338 / 0	-84.3	-84.3	0.07 (1)	6.25				
J-D	-397 / 0	-84.3	-84.3	0.04 (1)	6.25				
D-E	0 / 17	-84.3	-84.3	0.11 (1)	10.00				
B-G	0 / 291	-28.0	-28.0	0.08 (1)	10.00				
G-F	0 / 291	-28.0	-28.0	0.09 (1)	10.00				
F-I	0 / 291	-28.0	-28.0	0.09 (1)	10.00				
I-D	0 / 291	-28.0	-28.0	0.08 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.21")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.21")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.11 (D-E:1), BC=0.09 (F-G:1), WB=0.04 (C-F:2), SSI=0.09 (C-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2264 1958

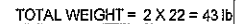
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (B) (INPUT = 0.90)
JSI METAL= 0.10 (D) (INPUT = 1.00)



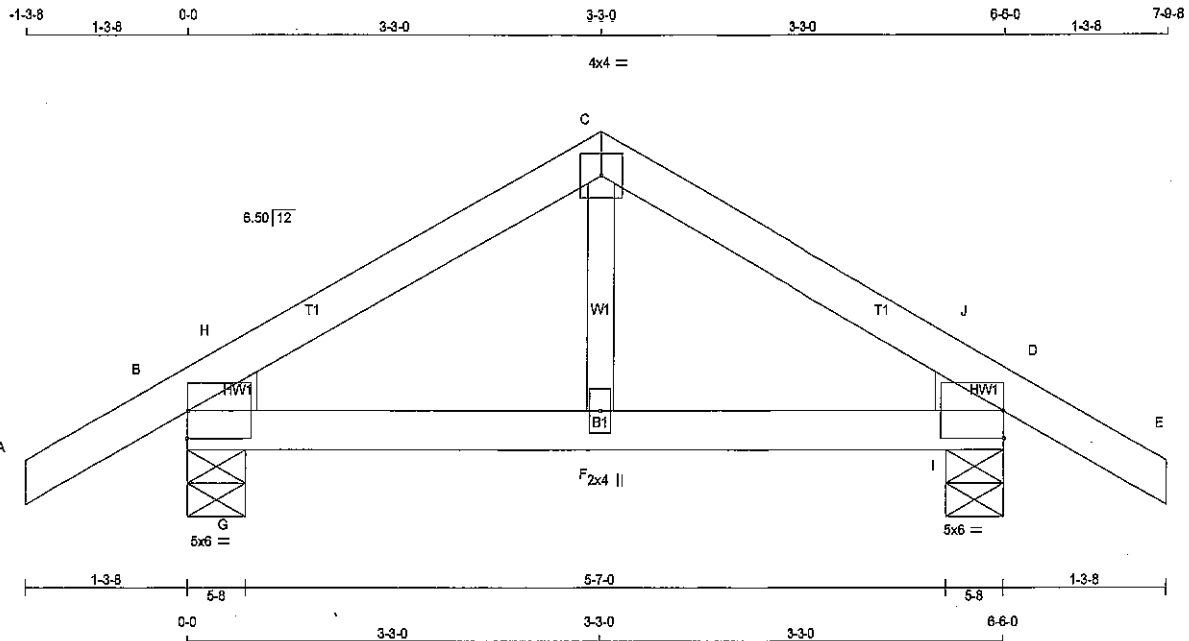
OWN NO. TAM 4510417
STRUCTURAL
COMPONENT ONLY



DWG NO. TAM 45205-17
STRUCTURAL
COMPONENT ONLY

JOB NAME 285861	TRUSS NAME T93	QUANTITY 2	PLY 1	JOB DESC. 43854 TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.030 S Oct 5 2016 MTEK Industries, Inc. Thu Sep 07 11:07:06 2017 Page 1
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Scale = 1:17.3

TOTAL WEIGHT = 2 X 22 = 44 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMBH1-I	MT20	5.0	6.0	
C	TTW-p	MT20	4.0	4.0	
D	TMBH1-I	MT20	5.0	6.0	
F	BMW-w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG	HEEL WEDGE
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX
B	480	0	480	0	0	5-8
D	480	0	480	0	0	5-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	377	236 / 0	68 / 0	0 / 0	0 / 0	73 / 0	0 / 0
D	377	236 / 0	68 / 0	0 / 0	0 / 0	73 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 MAX. (CSI (LC))	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED UNBRACED LENGTH (LC)	MAX. FACTORED (LC)
FR-TO		FROM TO		FR-TO			
A-B	0 / 17	-84.3 -84.3	0.11 (1)	10.00	F-C	0 / 185	0.04 (2)
B-H	-412 / 0	-84.3 -84.3	0.05 (1)	6.25	G-H	-34 / 113	0.00 (1)
H-C	-355 / 0	-84.3 -84.3	0.08 (1)	6.25	I-J	-34 / 113	0.00 (1)
C-J	-355 / 0	-84.3 -84.3	0.08 (1)	6.25			
J-D	-412 / 0	-84.3 -84.3	0.05 (1)	6.25			
D-E	0 / 17	-84.3 -84.3	0.11 (1)	10.00			
B-G	0 / 305	-28.0 -28.0	0.08 (1)	10.00			
G-F	0 / 305	-28.0 -28.0	0.10 (1)	10.00			
F-I	0 / 305	-28.0 -28.0	0.10 (1)	10.00			
I-D	0 / 305	-28.0 -28.0	0.08 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD	=	46.1	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.22")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.22")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.11 (A-B:1), BC=0.10 (F-G:1), WB=0.04 (C-F:2), SS=0.09 (C-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(FSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1667
	622	2284	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (D) (INPUT = 0.90)
JSI METAL= 0.10 (D) (INPUT = 1.00)



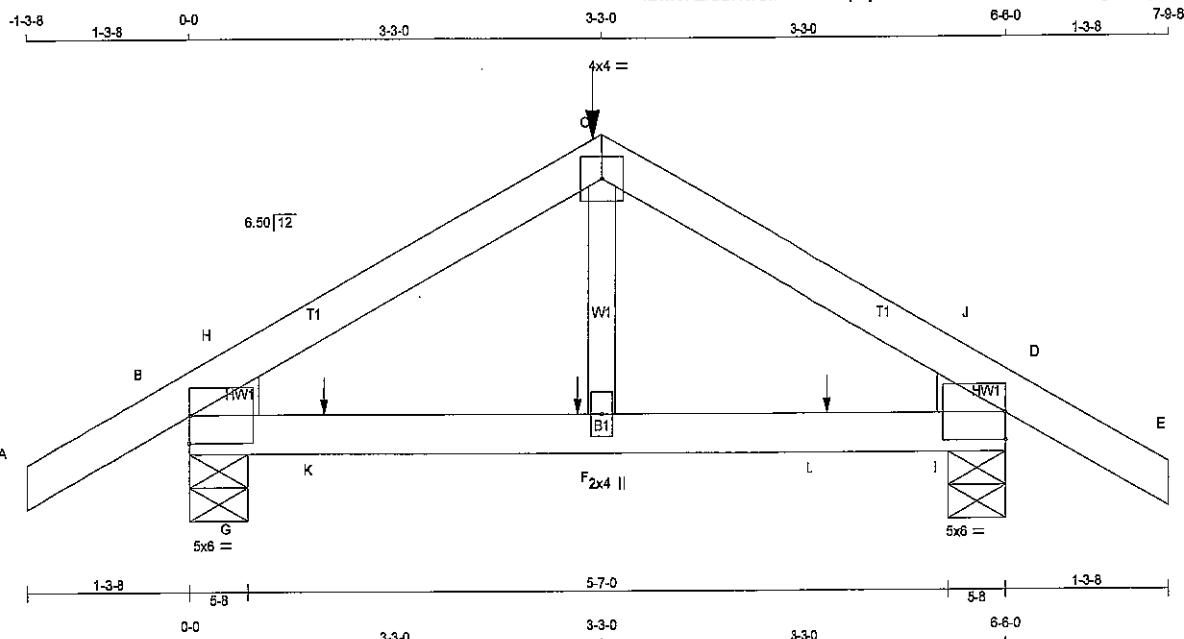
DWG NO. TAM 45706-87

STRUCTURAL
COMPONENT ONLY

JOB NAME 285861	TRUSS NAME T93Z	QUANTITY 1	PLY 1	JOB DESC. 43954	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Thu Sep 07 11:07:08 2017 Page 1
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Scale = 1:17.3

TOTAL WEIGHT = 22 lb

LUMBER

N. L. G. A. RULES

CHORDS SIZE

A - C 2x4 DRY No.2 SPF

C - E 2x4 DRY No.2 SPF

B - D 2x4 DRY No.2 SPF

ALL WEBS 2x3 DRY No.2 SPF

DRY; SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH-1	MT20	5.0	6.0		
C	TTW-p	MT20	4.0	4.0		
D	TMBH-1	MT20	5.0	6.0		
F	BMW-w	MT20	2.0	4.0		

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 2.4 lbs FACTORED DOWN AT 3-0-0 ON TOP CHORD, AND AT 1-0-12, AND AT 3-0-12, AND AT 5-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	HEEL
B	481	0	481	0	0	5-8	2x4 L
D	481	0	481	0	0	5-8	2x4 R

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	378	238 / 0	68 / 0	0 / 0	0 / 0	74 / 0	0 / 0
D	378	238 / 0	68 / 0	0 / 0	0 / 0	74 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LOAD LC1	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED LOAD LC1		
FR-TO		FROM	TO		FR-TO				
A-B	0 / 17	-84.3	-84.3	0.12 (1)	10.00	F-C	0 / 185	0.04 (2)	
B-H	-414 / 0	-84.3	-84.3	0.07 (1)	6.25	G-H	-33 / 114	0.00 (1)	
H-C	-357 / 0	-84.3	-84.3	0.10 (1)	6.25	I-J	-33 / 114	0.00 (1)	
C-J	-357 / 0	-84.3	-84.3	0.10 (1)	6.25				
J-D	-414 / 0	-84.3	-84.3	0.07 (1)	6.25				
D-E	0 / 17	-84.3	-84.3	0.12 (1)	10.00				
B-G	0 / 307	-28.0	-28.0	0.09 (1)	10.00				
G-K	0 / 307	-28.0	-28.0	0.11 (1)	10.00				
K-F	0 / 307	-28.0	-28.0	0.11 (1)	10.00				
F-L	0 / 307	-28.0	-28.0	0.11 (1)	10.00				
L-I	0 / 307	-28.0	-28.0	0.11 (1)	10.00				
I-D	0 / 307	-28.0	-28.0	0.09 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LOC1	MAX-	MAX+	FACE	DIR.	TYPE
C	3-0-0	-2	-2	-	FRONT	VERT	TOTAL
F	3-0-12	-	-	-	FRONT	VERT	TOTAL
K	1-0-12	-	-	-	FRONT	VERT	TOTAL
L	5-0-12	-	-	-	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.8	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN./C/C

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.22")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.22")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.01")

CSI: TC=0.12 (A-B:1), BC=0.11 (F-G:1), WB=0.04 (C-F:2), SS=0.10 (C-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	818	354	1657 822 2284 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (D) (INPUT = 0.90)
JSI METAL= 0.10 (D) (INPUT = 1.00)



DWG NO. TAM45107-17

STRUCTURAL

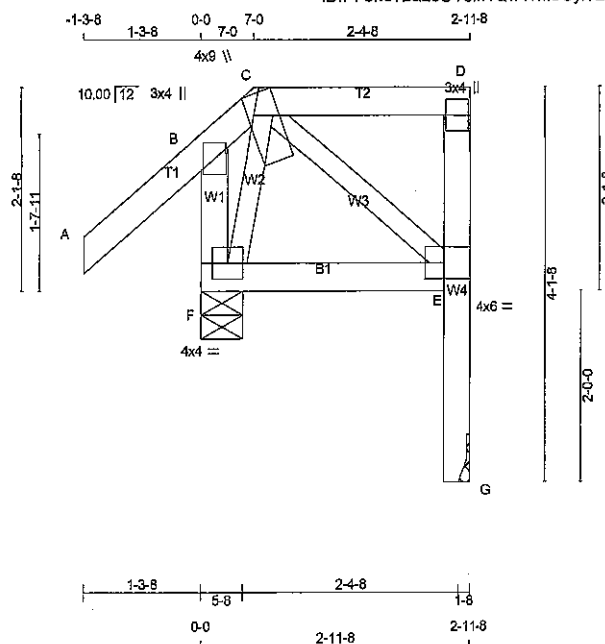
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	49954	DRWG NO.
285861	T102	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale: 1/2"=1'

TOTAL WEIGHT = 2 X 18 = 36 lb (M)

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
G - D	2x4	DRY	No.2	SPF	
F - B	2x4	DRY	No.2	SPF	
F - E	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0			
C	TTWW+m	MT20	4.0	8.0	Edge	0.75	
D	TMV+p	MT20	3.0	4.0			
E	BVMW-l	MT20	4.0	6.0	2.00	2.50	
F	BVMW-l	MT20	4.0	4.0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
G	139	0	139	0
F	310	0	310	0

UNFACTORED REACTIONS	1ST LCASE	MAX/MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
G	118	59 / -17	31 / 0
F	234	163 / 0	31 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING	CHORDS	WEBS
TOTAL LOAD CASES: (5)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
	VERT. LOAD (PLF)	MAX. MEMB. LENGTH FR-TO
	LC1 (LC)	MAX. FORCE (LBS)
	CS1 (LC)	MAX. CS1 (LC)
FR-TO	FROM TO	
A-B	0 / 37	-84.3 -84.3 0.12 (1)
B-C	-86 / 0	-84.3 -84.3 0.11 (1)
C-D	0 / 0	-84.3 -84.3 0.08 (1)
G-E	-139 / 0	0.0 0.0 0.02 (1)
E-D	-99 / 0	0.0 0.0 0.01 (1)
F-B	-276 / 0	0.0 0.0 0.03 (1)
F-E	-33 / 0	-28.0 -28.0 0.07 (3)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:	TOP CH.	LL	PSF
		25.6	PSF
	DL	3.0	PSF
BOT CH.	LL	10.5	PSF
	DL	7.0	PSF
TOTAL LOAD		48.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.01")

CSI: TC=0.12 (A-B:1), BC=0.07 (E-F:3), WB=0.02 (C-F:5), SSI=0.09 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 622	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.16 (F) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)



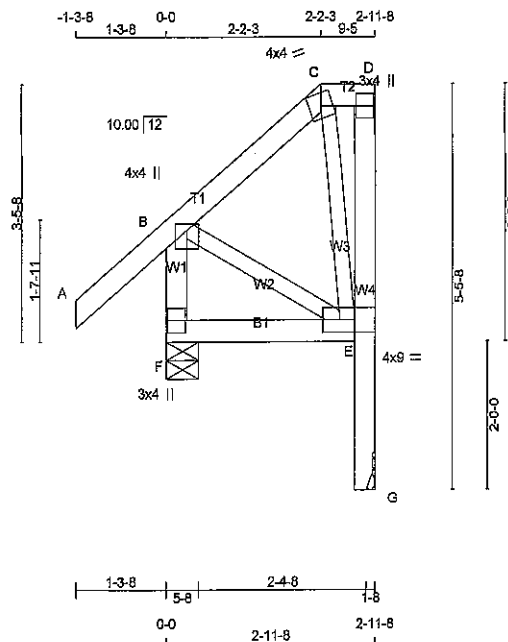
DWG NO. TAM 4510817
STRUCTURAL
COMPONENT ONLY

JOB NAME 285861	TRUSS NAME T103	QUANTITY 2	PLY 1	JOB DESC. 43984 TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1:31.2

TOTAL WEIGHT = 2 X 21 = 42 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
G - D	2x4	DRY	No.2	SPF
F - B	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW-m	MT20	4.0	4.0		
D	TMV+p	MT20	3.0	4.0		
E	BVMWW-I	MT20	4.0	9.0	2.00	3.00
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
G	188	0	166	0
F	283	0	283	0

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	136	76 / 0	31 / 0	0 / 0	0 / 0	30 / 0	0 / 0
F	216	147 / 0	31 / 0	0 / 0	0 / 0	38 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 37	-84.3	-84.3	0.12 (1)	10.00	C-E	-106 / 0
B-C	-33 / 0	-84.3	-84.3	0.07 (1)	6.25	B-E	0 / 28
C-D	0 / 0	-84.3	-84.3	0.01 (1)	10.00		
G-E	-188 / 0	0.0	0.0	0.02 (1)	7.81		
E-D	-33 / 0	0.0	0.0	0.01 (1)	7.81		
F-B	-241 / 0	0.0	0.0	0.03 (1)	7.81		
F-E	0 / 0	-28.0	-28.0	0.07 (3)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 3.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.12 (A-B:1), BC=0.07 (E-F:3), WB=0.02 (C-E:1), SSI=0.07 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1887
	822	2284	1658

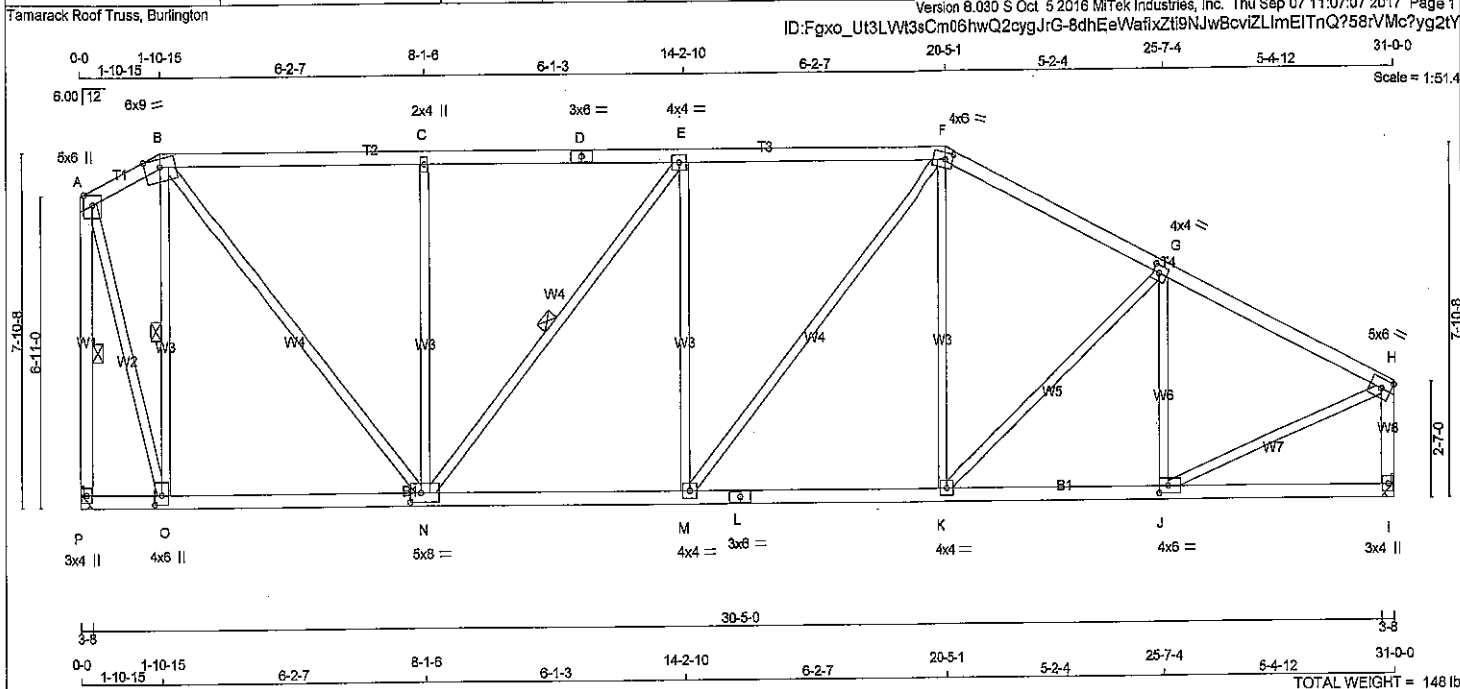
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)



DWG NO. TAM4309-17
STRUCTURAL
COMPONENT ONLY



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY No.2	SPF
B - D	2x4	DRY No.2	SPF
D - F	2x4	DRY No.2	SPF
F - H	2x4	DRY No.2	SPF
P - A	2x4	DRY No.2	SPF
I - H	2x4	DRY No.2	SPF
P - L	2x4	DRY No.2	SPF
L - I	2x4	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW+p	MT20	5.0	6.0	Edge
B	TTWW-m	MT20	6.0	9.0	Edge
C	TMW+w	MT20	2.0	4.0	
D	TS-t	MT20	3.0	6.0	
E	TMWW-t	MT20	4.0	4.0	
F	TTWW-m	MT20	4.0	6.0	1.75 2.25
G	TMWW-t	MT20	4.0	4.0	2.00 1.75
H	TMWW-t	MT20	5.0	6.0	Edge
I	BMV1+p	MT20	3.0	4.0	
J	BMWW-t	MT20	4.0	6.0	2.00 2.50
K	BMWW-t	MT20	4.0	4.0	
L	BS-t	MT20	3.0	6.0	
M	BMWW-t	MT20	4.0	4.0	
N	BMWW-t	MT20	5.0	6.0	2.50 3.00
O	BMWW-t	MT20	4.0	6.0	2.50 2.00
P	BMV1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG UPLIFT		REQD BRG IN-SX	
JT	VERT	HORZ	DOWN	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
P	1740	0	1740	0	0	0	0	HANGER BY OTHERS	MIN. SEAT SIZE: 3-8
I	1740	0	1740	0	0	0	0	HANGER BY OTHERS	MIN. SEAT SIZE: 3-8

UNFACTORED REACTIONS		1ST LCASE		MAX/MIN. COMPONENT REACTIONS		SNOW		LIVE		PERM. LIVE		WIND		DEAD		SOIL	
JT	COMBINED	VERT	HORZ	DOWN	HORZ	DOWN	HORZ	DOWN	HORZ	DOWN	HORZ	DOWN	HORZ	DOWN	HORZ	DOWN	HORZ
P	1429	793 / 0	326 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	310 / 0	0 / 0	0 / 0	0 / 0
I	1429	793 / 0	326 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	310 / 0	0 / 0	0 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.34 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF B-O, A-P, E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		MEMB.		MAX. FACTORED FORCE (LBS)		FACTORED VERT. LOAD (PLF)		MAX. FACTORED VERT. LOAD (PLF)		MAX. FACTORED VERT. LOAD (PLF)		MAX. FACTORED VERT. LOAD (PLF)		MAX. FACTORED VERT. LOAD (PLF)		MAX. FACTORED VERT. LOAD (PLF)	
FR-TO	FROM	TO	FROM	TO	FROM	TO	FROM	TO	FROM	TO	FROM	TO	FROM	TO	FROM	TO	FROM
A-B	-492 / 0	-84.3	-84.3	0.06 (1)	6.25	0-B	-1300 / 0	0.51 (1)	0.51 (1)	0.51 (1)	0.51 (1)	0.51 (1)	0.51 (1)	0.51 (1)	0.51 (1)	0.51 (1)	0.51 (1)
B-C	-1427 / 0	-84.3	-84.3	0.51 (1)	4.83	K-F	0 / 278	0.06 (2)	0.06 (2)	0.06 (2)	0.06 (2)	0.06 (2)	0.06 (2)	0.06 (2)	0.06 (2)	0.06 (2)	0.06 (2)
C-D	-1427 / 0	-84.3	-84.3	0.51 (1)	4.82	K-G	-28 / 7	0.03 (1)	0.03 (1)	0.03 (1)	0.03 (1)	0.03 (1)	0.03 (1)	0.03 (1)	0.03 (1)	0.03 (1)	0.03 (1)
D-E	-1427 / 0	-84.3	-84.3	0.51 (1)	4.82	J-G	-528 / 0	0.22 (1)	0.22 (1)	0.22 (1)	0.22 (1)	0.22 (1)	0.22 (1)	0.22 (1)	0.22 (1)	0.22 (1)	0.22 (1)
E-F	-1819 / 0	-84.3	-84.3	0.55 (1)	4.34	A-O	0 / 1504	0.34 (1)	0.34 (1)	0.34 (1)	0.34 (1)	0.34 (1)	0.34 (1)	0.34 (1)	0.34 (1)	0.34 (1)	0.34 (1)
F-G	-1825 / 0	-84.3	-84.3	0.49 (1)	4.41	J-H	0 / 1768	0.40 (1)	0.40 (1)	0.40 (1)	0.40 (1)	0.40 (1)	0.40 (1)	0.40 (1)	0.40 (1)	0.40 (1)	0.40 (1)
G-H	-1799 / 0	-84.3	-84.3	0.49 (1)	4.44	M-F	0 / 326	0.07 (1)	0.07 (1)	0.07 (1)	0.07 (1)	0.07 (1)	0.07 (1)	0.07 (1)	0.07 (1)	0.07 (1)	0.07 (1)
P-A	-1740 / 0	0.0	0.0	0.39 (1)	5.05	B-N	0 / 1587	0.38 (1)	0.38 (1)	0.38 (1)	0.38 (1)	0.38 (1)	0.38 (1)	0.38 (1)	0.38 (1)	0.38 (1)	0.38 (1)
I-H	-1877 / 0	0.0	0.0	0.23 (1)	6.41	M-E	-80 / 195	0.09 (1)	0.09 (1)	0.09 (1)	0.09 (1)	0.09 (1)	0.09 (1)	0.09 (1)	0.09 (1)	0.09 (1)	0.09 (1)
P-O	0 / 0	-28.0	-28.0	0.15 (3)	10.00	N-C	-565 / 0	0.66 (1)	0.66 (1)	0.66 (1)	0.66 (1)	0.66 (1)	0.66 (1)	0.66 (1)	0.66 (1)	0.66 (1)	0.66 (1)
O-N	0 / 421	-28.0	-28.0	0.26 (2)	10.00	N-E	-625 / 0	0.42 (1)	0.42 (1)	0.42 (1)	0.42 (1)	0.42 (1)	0.42 (1)	0.42 (1)	0.42 (1)	0.42 (1)	0.42 (1)
N-M	0 / 1819	-28.0	-28.0	0.42 (2)	10.00												
M-L	0 / 1612	-28.0	-28.0	0.40 (2)	10.00												
L-K	0 / 1612	-28.0	-28.0	0.40 (2)	10.00												
K-J	0 / 1829	-28.0	-28.0	0.37 (2)	10.00												
J-I	0 / 0	-28.0	-28.0	0.18 (3)	10.00												

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/380 (1.03")
CALCULATED VERT. DEFL. (LL) = L/999 (0.11")
ALLOWABLE DEFL. (TL) = L/380 (1.03")
CALCULATED VERT. DEFL. (TL) = L/999 (0.18")

CSI: TC=0.55 (E-F:1), BC=0.42 (M-N:2), WB=0.86 (C-N:1), SSI=0.24 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (N) (INPUT = 0.90)
JSI METAL= 0.47 (H) (INPUT = 1.00)



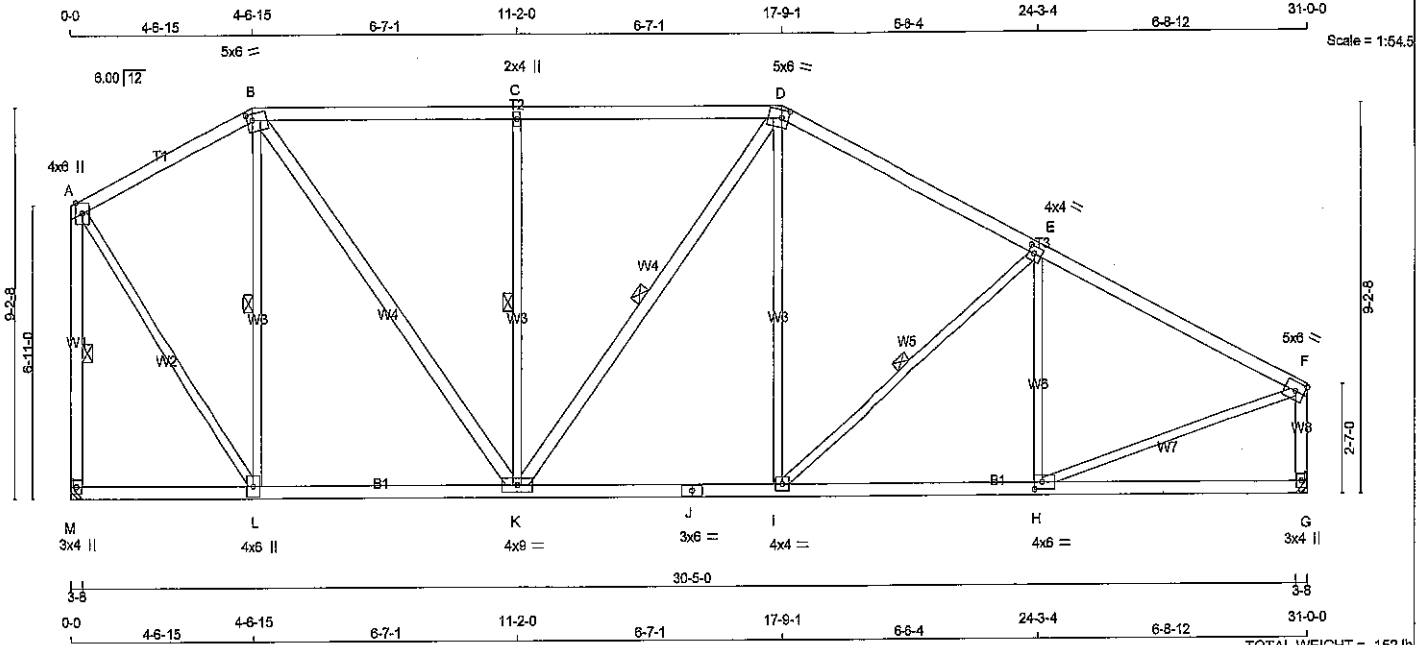
AWB NO. TAM 4510-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
285861	T180	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 152 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2
B - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
M - A	2x4	DRY	No.2
G - F	2x4	DRY	No.2
M - J	2x4	DRY	No.2
J - G	2x4	DRY	No.2

ALL WEBS EXCEPT			
CHORDS	SIZE	LUMBER	DESCR.
B - K	2x4	DRY	No.2
K - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
B	TTWW-m	MT20	5.0	6.0	1.75	1.75
C	TMW+w	MT20	2.0	4.0		
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0	2.00	1.75
F	TMVW-t	MT20	5.0	6.0	Edge	
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	4.0	6.0	2.00	2.25
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	9.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
M	1740	0	1740	0	0
G	1740	0	1740	0	0

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	MAX/MIN. LIVE	PERM. LIVE	WIND	DEAD	SOIL
M	1429	793 / 0	326 / 0	0 / 0	0 / 0	310 / 0	0 / 0
G	1429	793 / 0	326 / 0	0 / 0	0 / 0	310 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.24 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF B-L, C-K, D-K, E-I, A-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	LC1 MAX (LC)
FR-TO		FROM TO		FR-TO			
A-B	-878 / 0	-84.3 -84.3	0.24 (1)	L-B	-932 / 0	6.25	0.52 (1)
B-C	-1455 / 0	-84.3 -84.3	0.51 (1)	B-K	0 / 1146	4.79	0.18 (1)
C-D	-1455 / 0	-84.3 -84.3	0.51 (1)	K-C	-688 / 0	4.79	0.38 (1)
D-E	-1696 / 0	-84.3 -84.3	0.54 (1)	K-D	-75 / 0	4.48	0.05 (2)
E-F	-1900 / 0	-84.3 -84.3	0.58 (1)	I-D	0 / 446	4.24	0.10 (2)
M-A	-1893 / 0	0.0 0.0	0.38 (1)	J-E	-312 / 0	5.11	0.18 (1)
G-F	-1683 / 0	0.0 0.0	0.22 (1)	H-E	-375 / 79	6.43	0.21 (1)
				A-L	0 / 1358		0.31 (1)
				H-F	0 / 1622		0.41 (1)
M-L	0 / 0	-28.0 -28.0	0.21 (2)			10.00	
L-K	0 / 773	-28.0 -28.0	0.33 (2)			10.00	
K-J	0 / 1493	-28.0 -28.0	0.41 (2)			10.00	
J-I	0 / 1493	-28.0 -28.0	0.41 (2)			10.00	
I-H	0 / 1726	-28.0 -28.0	0.51 (2)			10.00	
H-G	0 / 0	-28.0 -28.0	0.31 (3)			10.00	

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 25.6	PSF
	DL	= 3.0	PSF
BOT CH.	LL	= 10.5	PSF
	DL	= 7.0	PSF
TOTAL LOAD	= 46.1	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL)= L/999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL)= L/999 (0.16")

CSI: TC=0.58 (E-F:1), BC=0.51 (H-I:2), WB=0.52 (B-L:1), SI=0.27 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

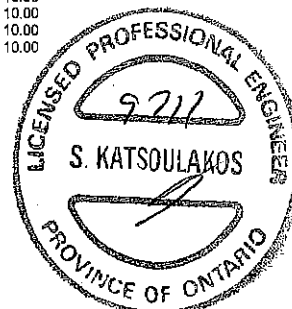
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(FSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (F) (INPUT = 0.80)
JSI METAL= 0.50 (F) (INPUT = 1.00)

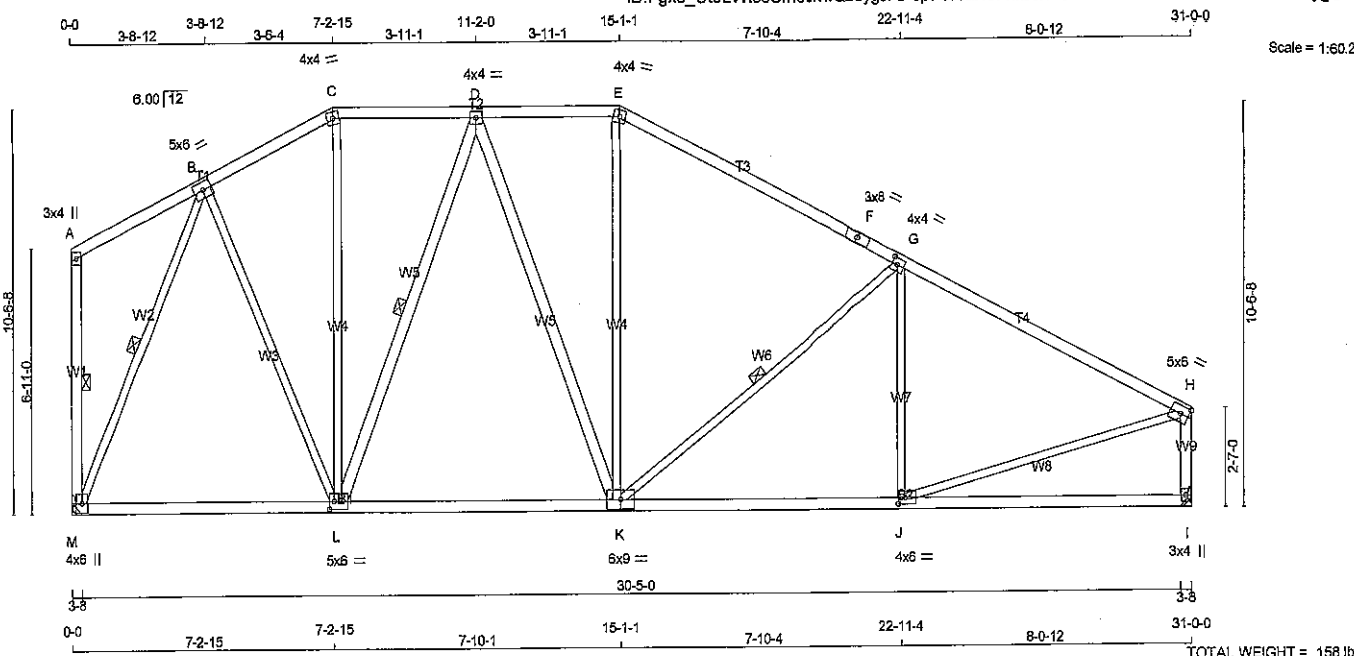


DRW NO. TAM 4511-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
285861	T181	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:Fgxo_Ut3LWt3sCm08hwQ2cygJrG-cpFcssaH3FhkJJxUu78EmuOGeaAWoQENVEw9Ryg2tX



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
M - A	2x4	DRY	No.2
I - H	2x4	DRY	No.2
M - K	2x4	DRY	No.2
K - I	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
L - D	2x4	DRY	No.2
D - K	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
A	TMV+p	MT20	3.0	4.0
B	TMWW-t	MT20	5.0	6.0
C	TTW-m	MT20	4.0	4.0
D	TMWW-t	MT20	4.0	4.0
E	TTW-m	MT20	4.0	4.0
F	TS-t	MT20	3.0	8.0
G	TMWW-t	MT20	4.0	4.0
H	TMWW-t	MT20	5.0	6.0
I	BMV1+p	MT20	3.0	4.0
J	BMWW-t	MT20	4.0	6.0
K	BSWW-t	MT20	6.0	9.0
L	BMWW-t	MT20	5.0	6.0
M	BMWW-t	MT20	4.0	6.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
M	1740	0	1740	0	0
I	1740	0	1740	0	0

UNFACTORED REACTIONS		1ST LCASE	MAX /MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
M	1429	793 / 0	326 / 0
I	1429	793 / 0	326 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.61 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.
1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-L, G-K, A-M, B-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING		CHORDS		WEBS	
TOTAL LOAD CASES: (4)		MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)
FR-TO				FR-TO	
A-B	0 / 15	-84.3	-84.3 0.17 (1)	B-L	0 / 741
B-C	-1095 / 0	-84.3	-84.3 0.14 (1)	L-C	0 / 263
C-D	-967 / 0	-84.3	-84.3 0.17 (1)	L-D	-727 / 0
D-E	-1328 / 0	-84.3	-84.3 0.18 (1)	D-K	0 / 299
E-F	-1522 / 0	-84.3	-84.3 0.80 (1)	K-E	0 / 342
F-G	-1522 / 0	-84.3	-84.3 0.80 (1)	K-G	-577 / 0
G-H	-1952 / 0	-84.3	-84.3 0.89 (1)	J-G	-250 / 194
H-A	-121 / 0	0.0	0.0 0.03 (1)	M-B	-1877 / 0
I-H	-1649 / 0	0.0	0.0 0.22 (1)	J-H	0 / 1847
M-L	0 / 869	-28.0	-28.0 0.47 (2)		
L-K	0 / 1221	-28.0	-28.0 0.54 (2)		
K-J	0 / 1778	-28.0	-28.0 0.85 (2)		
J-I	0 / 0	-28.0	-28.0 0.45 (3)		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN/GC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/999 (0.28")

CSI: TC=0.89 (G-H:1), BC=0.65 (J-K:2), WB=0.97 (B-M:1), SI=0.29 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 622 2284 1658

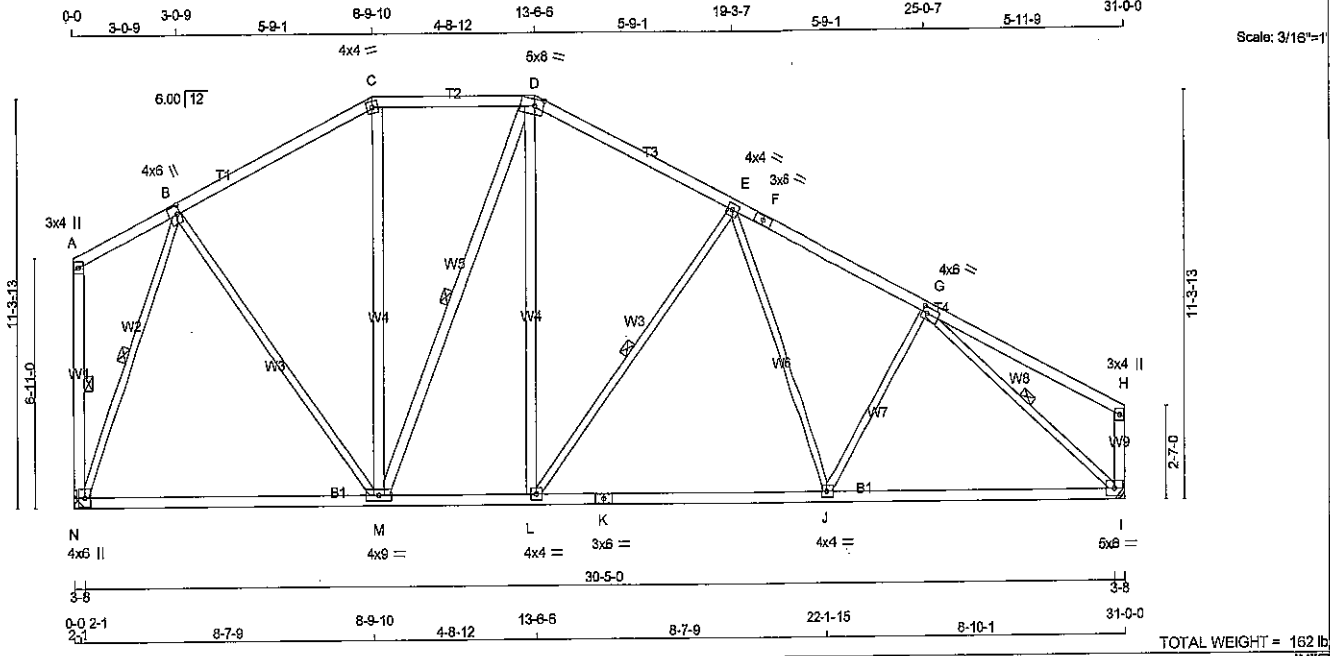
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (J) (INPUT = 0.80)
JSI METAL= 0.52 (H) (INPUT = 1.00)



DWG NO. TAM 45712-17
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - A	2x4	DRY	No.2
I - H	2x4	DRY	No.2
N - K	2x4	DRY	No.2
K - I	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
M - C	2x4	DRY	No.2
M - D	2x4	DRY	No.2
L - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW+t	MT20	4.0	6.0	3.00	1.00
C	TTW-m	MT20	4.0	4.0		
D	TTWW-m	MT20	5.0	8.0	2.25	3.00
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	6.0	2.00	1.75
H	TMV+p	MT20	3.0	4.0		
I	BMWW-t	MT20	5.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMWW-t	MT20	4.0	9.0		
N	BMVW1+p	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
N	1740	0	1740	0		
I	1740	0	1740	0		

MIN. SEAT SIZE: 3-8
HANGER BY OTHERS

UNFACTORED REACTIONS

JT	MAX./MIN. COMPONENT REACTIONS					
	1ST CASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
N	1429	793 / 0	326 / 0	0 / 0	0 / 0	310 / 0
I	1429	793 / 0	326 / 0	0 / 0	0 / 0	310 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF B-N, D-M, E-L, A-N, G-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		MAX. UNBRAC	MAX. FACTORED	MEMB.	WEBS		MAX. FACTORED
	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX. (LC)				MAX. FORCE (LBS)	MAX. (LC)	
FR-TO							FR-TO			
A-B	0 / 36	-84.3	-84.3	0.34 (1)	10.00		N-B	-1697 / 0	0.86 (1)	
B-C	-1159 / 0	-84.3	-84.3	0.34 (1)	5.51		B-M	0 / 720	0.16 (1)	
C-D	-1020 / 0	-84.3	-84.3	0.25 (1)	5.90		M-C	0 / 231	0.04 (3)	
D-E	-1368 / 0	-84.3	-84.3	0.36 (1)	5.16		M-D	-500 / 0	0.40 (1)	
E-F	-1882 / 0	-84.3	-84.3	0.36 (1)	4.58		L-D	0 / 671	0.11 (1)	
F-G	-1862 / 0	-84.3	-84.3	0.36 (1)	4.58		L-E	-653 / 0	0.48 (1)	
G-H	0 / 21	-84.3	-84.3	0.36 (1)	10.00		E-J	0 / 348	0.08 (2)	
N-A	-47 / 0	0.0	0.0	0.01 (1)	6.25		J-G	0 / 228	0.05 (3)	
I-H	-201 / 0	0.0	0.0	0.03 (1)	7.81		G-I	-2174 / 0	0.88 (1)	
N-M	0 / 596	-28.0	-28.0	0.53 (2)	10.00					
M-L	0 / 1212	-28.0	-28.0	0.61 (2)	10.00					
L-K	0 / 1587	-28.0	-28.0	0.77 (2)	10.00					
K-J	0 / 1587	-28.0	-28.0	0.77 (2)	10.00					
J-I	0 / 1627	-28.0	-28.0	0.77 (2)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. GIG

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/900 (0.28")
ALLOWABLE DEFL.(TL) = L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/858 (0.43")

CSI: TC=0.38 (G-H:1), BC=0.77 (I-J:2), WB=0.88 (G-I:1), SSI=0.21 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

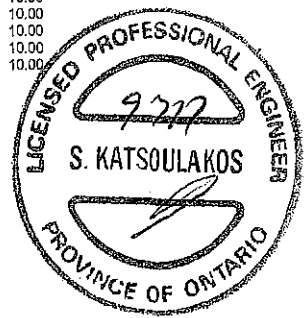
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 518 354 1667 822 2284 1656

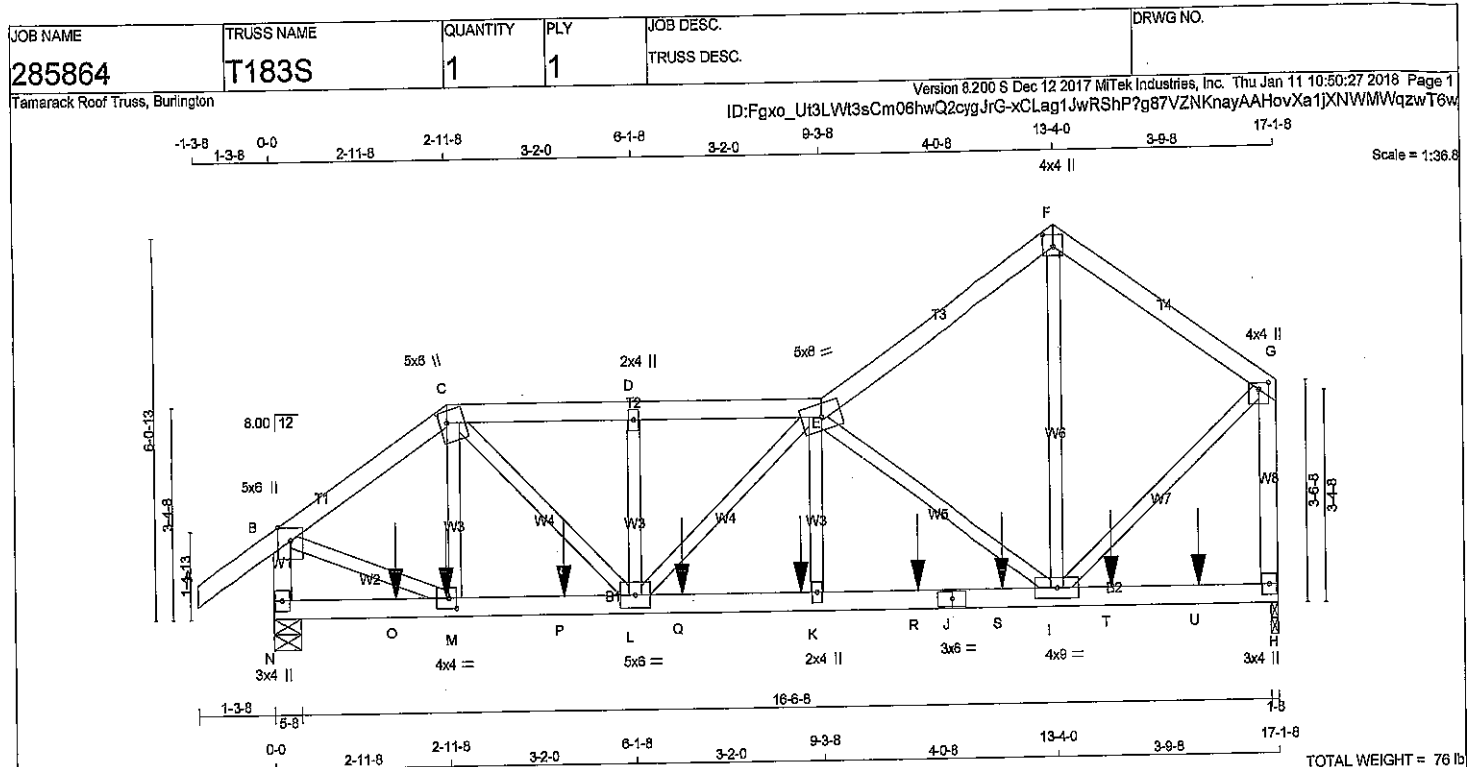
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (I) (INPUT= 0.90)
JSI METAL= 0.58 (K) (INPUT= 1.00)



DRW NO. YAM 45113-17
STRUCTURAL COMPONENT ONLY



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
E - F	2x4	DRY	No.2	SPF	
F - G	2x4	DRY	No.2	SPF	
N - B	2x4	DRY	No.2	SPF	
H - G	2x4	DRY	No.2	SPF	
N - J	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMVW+w	MT20	2.0	4.0		
E	TTWWW+m	MT20	6.0	8.0		
F	TTW+p	MT20	4.0	4.0	2.25	2.00
G	TMVW+p	MT20	4.0	4.0	1.25	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMVW-w	MT20	4.0	9.0		
J	BS-t	MT20	3.0	6.0		
K	BMVW-w	MT20	2.0	4.0		
L	BMVW-w	MT20	5.0	6.0		
M	BMVW-t	MT20	4.0	4.0	2.00	1.50
N	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 14.8 lbs FACTORED DOWN AT 2-0-12, 14.8 lbs FACTORED DOWN AT 2-11-0, 14.8 lbs FACTORED DOWN AT 4-11-0, 14.8 lbs FACTORED DOWN AT 6-11-0, 14.8 lbs FACTORED DOWN AT 8-11-0, 110.7 lbs FACTORED DOWN AT 10-10-12, 138.0 lbs FACTORED DOWN AT 12-4-12, AND 138.0 lbs FACTORED DOWN AT 14-3-4, AND 110.7 lbs FACTORED DOWN AT 15-9-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD IN-SX
JT VERT	HORZ	DOWN	HORZ	UPLIFT
N	1216	0	1216	0
H	1381	0	1381	0

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
N	986	568 / 0	211 / 0	0 / 0	0 / 0	207 / 0	0 / 0
H	1092	848 / 0	221 / 0	0 / 0	0 / 0	223 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.15 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

FR-TO	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	CS1 (LC)	UNBRACED LENGTH	FR-TO	WEBS MAX. FACTORED FORCE (LBS)	MAX	CS1 (LC)
A-B	0 / 32	-84.3	-84.3	0.12 (1)	10.00	M-C	-200 / 36	0.04 (1)	
B-C	-1107 / 0	-84.3	-84.3	0.16 (1)	5.81	C-L	0 / 833	0.21 (1)	
C-D	-1510 / 0	-84.3	-84.3	0.16 (1)	5.15	L-D	-308 / 0	0.07 (1)	
D-E	-1510 / 0	-84.3	-84.3	0.16 (1)	5.15	E-E	-281 / 0	0.09 (1)	
E-F	-866 / 0	-84.3	-84.3	0.28 (1)	6.18	K-E	0 / 237	0.08 (2)	
F-G	-867 / 0	-84.3	-84.3	0.25 (1)	6.23	E-I	-1244 / 0	0.55 (1)	
N-B	-1181 / 0	0.0	0.0	0.13 (1)	7.31	I-F	0 / 630	0.16 (1)	
H-G	-1254 / 0	0.0	0.0	0.27 (1)	7.14	B-M	0 / 874	0.24 (1)	
						I-G	0 / 947	0.23 (1)	
N-O	0 / 0	-28.0	-28.0	0.08 (2)	10.00				
O-M	0 / 0	-28.0	-28.0	0.08 (2)	10.00				
M-P	0 / 812	-28.0	-28.0	0.21 (1)	10.00				
P-L	0 / 812	-28.0	-28.0	0.21 (1)	10.00				
L-Q	0 / 1718	-28.0	-28.0	0.39 (1)	10.00				
Q-K	0 / 1718	-28.0	-28.0	0.39 (1)	10.00				
K-R	0 / 1713	-28.0	-28.0	0.58 (1)	10.00				
R-J	0 / 1713	-28.0	-28.0	0.58 (1)	10.00				
J-S	0 / 1713	-28.0	-28.0	0.58 (1)	10.00				
S-I	0 / 1713	-28.0	-28.0	0.58 (1)	10.00				
I-T	0 / 0	-28.0	-28.0	0.29 (1)	10.00				
T-U	0 / 0	-28.0	-28.0	0.29 (1)	10.00				
U-H	0 / 0	-28.0	-28.0	0.29 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TOTAL
K	8-11-0	-8	-15	---	FRONT	VERT	TOTAL
M	2-11-0	-8	-15	---	FRONT	VERT	TOTAL
O	2-0-12	-8	-15	---	FRONT	VERT	TOTAL
P	4-11-0	-8	-15	---	FRONT	VERT	TOTAL
Q	6-11-0	-8	-15	---	FRONT	VERT	TOTAL
R	10-10-12	-111	-111	---	FRONT	VERT	TOTAL
S	12-4-12	-138	-138	---	FRONT	VERT	TOTAL
T	14-3-4	-138	-138	---	FRONT	VERT	TOTAL
U	15-9-4	-111	-111	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.8	PSF
BRG	DL = 3.0	PSF
BOT CH.	LL = 10.5	PSF
DL	DL = 7.0	PSF
TOTAL LOAD	= 46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/380$ (0.57")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.08")
ALLOWABLE DEFL.(TL) = $L/380$ (0.57")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.10")

CS1: TC=0.28/1.00 (E-F:1), BC=0.59/1.00 (I-K:1),
WB=0.55/1.00 (E-I:1), SSI=0.21/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

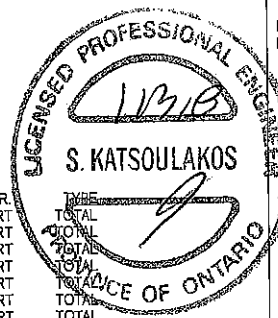
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1887
	822	2284	1855

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.90 (C) (INPUT = 0.90)
SI METAL= 0.82 (J) (INPUT = 1.00)

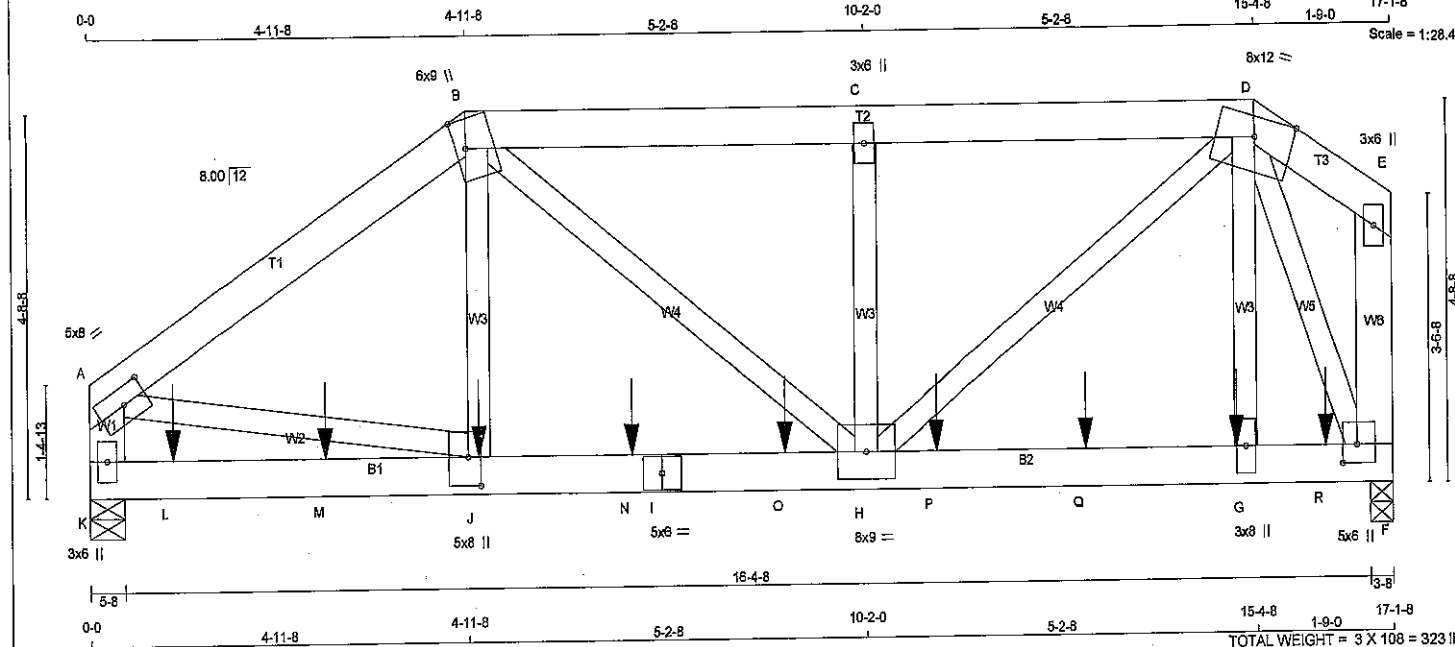


DWG NO. YAM 3336-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285864	T184	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
A - B	2x6	DRY No.2	SPF
B - D	2x6	DRY No.2	SPF
D - E	2x6	DRY No.2	SPF
K - A	2x6	DRY No.2	SPF
F - E	2x6	DRY No.2	SPF
K - I	2x6	DRY 2100F 1.8E	SPF
I - F	2x6	DRY 2100F 1.8E	SPF

ALL WEBS 2x4 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-B 2	12	TOP
B-D 2	12	TOP
D-E 2	12	TOP
K-A 2	12	TOP
F-E 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
K-I 2	8	SIDE (494.5)
I-F 2	8	SIDE (582.9)
WEBS : (0.122"x3") SPIRAL NAILS		
2x4 1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A TMVW-I	MT20	5.0	8.0	2.50	3.75
B TTWW+m	MT20	6.0	9.0	4.25	1.50
C TMW+w	MT20	3.0	6.0		
D TTWWW+m	MT20	8.0	12.0	Edge	
E TMV+p	MT20	3.0	6.0		
F BMVW1+p	MT20	5.0	8.0	2.75	2.25
G BMW+w	MT20	3.0	6.0		
H BMWWW-I	MT20	8.0	9.0		
I BS-I	MT20	5.0	6.0		
J BMVW+H	MT20	5.0	8.0	4.25	2.00
K BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
K	8529	0	8529	0	5-8	5-8
F	9145	0	9145	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
K	6912	3985 / 0	1476 / 0	0 / 0	0 / 0	1451 / 0	0 / 0
F	7444	4236 / 0	1626 / 0	0 / 0	0 / 0	1581 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

FR-TO	CHORDS		FACTORED		MAX. UNBRACED LENGTH	FR-TO	WEBS		MAX. UNBRACED LENGTH
	MAX. FACTORED	FORCE (LBS)	VERT. (PLF)	LC1 MAX (CSI (LC))			MAX. FACTORED	FORCE (LBS)	
A-B	-9228 / 0	-84.3	-84.3	0.11 (1)	4.75	J-B	0 / 3942	0.21 (1)	
B-C	-8719 / 0	-84.3	-84.3	0.10 (1)	4.86	B-H	0 / 1185	0.08 (1)	
C-D	-8719 / 0	-84.3	-84.3	0.10 (1)	4.86	H-C	-381 / 31	0.03 (1)	
D-E	0 / 0	-84.3	-84.3	0.01 (1)	10.00	H-D	0 / 8457	0.35 (1)	
K-A	-6698 / 0	0.0	0.0	0.14 (1)	6.84	G-D	0 / 4205	0.23 (1)	
F-E	-74 / 0	0.0	0.0	0.00 (1)	7.81	A-J	0 / 7779	0.42 (1)	
						D-F	-9300 / 0	0.66 (1)	
K-L	0 / 0	-28.0	-28.0	0.24 (1)	10.00				
L-M	0 / 0	-28.0	-28.0	0.24 (1)	10.00				
M-J	0 / 0	-28.0	-28.0	0.24 (1)	10.00				
J-N	0 / 7798	-28.0	-28.0	0.35 (1)	10.00				
N-I	0 / 7798	-28.0	-28.0	0.35 (1)	10.00				
I-O	0 / 7798	-28.0	-28.0	0.35 (1)	10.00				
O-H	0 / 7798	-28.0	-28.0	0.35 (1)	10.00				
H-P	0 / 3704	-28.0	-28.0	0.28 (1)	10.00				
P-Q	0 / 3704	-28.0	-28.0	0.28 (1)	10.00				
Q-G	0 / 3704	-28.0	-28.0	0.28 (1)	10.00				
G-R	0 / 3559	-28.0	-28.0	0.17 (1)	10.00				
R-F	0 / 3559	-28.0	-28.0	0.17 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.
G	15-1-4	-1712	-1712		BACK	VERT
J	5-1-4	-1826	-1826		BACK	VERT
L	1-1-4	-1826	-1826		BACK	VERT
M	3-1-4	-1826	-1826		BACK	VERT
N	7-1-4	-1712	-1712		BACK	VERT
O	9-1-4	-1712	-1712		BACK	VERT
P	11-1-4	-1712	-1712		BACK	VERT
Q	13-1-4	-1712	-1712		BACK	VERT
R	16-3-4	-1714	-1714		BACK	VERT

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 3.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 48.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.57")
CALCULATED VERT. DEFL.(LL)= L/999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.57")
CALCULATED VERT. DEFL.(TL)= L/999 (0.11")

CSI: TC=0.14/1.00 (A-K:1), BC=0.39/1.00 (H-J:1), WB=0.66/1.00 (D-F:1), SS=0.47/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	618	354	1667

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (J) (INPUT = 0.90)
JSI METAL= 0.68 (J) (INPUT = 1.00)

DWG NO. YAM 3337-18
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285864	T184	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1826.2 lbs FACTORED DOWN AT 1-1-4, 1826.2 lbs FACTORED DOWN AT 3-1-4, 1826.2 lbs FACTORED DOWN AT 5-1-4, 1711.9 lbs FACTORED DOWN AT 7-1-4, 1711.9 lbs FACTORED DOWN AT 9-1-4, 1711.9 lbs FACTORED DOWN AT 11-1-4, 1711.9 lbs FACTORED DOWN AT 13-1-4, AND 1711.9 lbs FACTORED DOWN AT 15-1-4, AND 1714.0 lbs FACTORED DOWN AT 16-3-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.



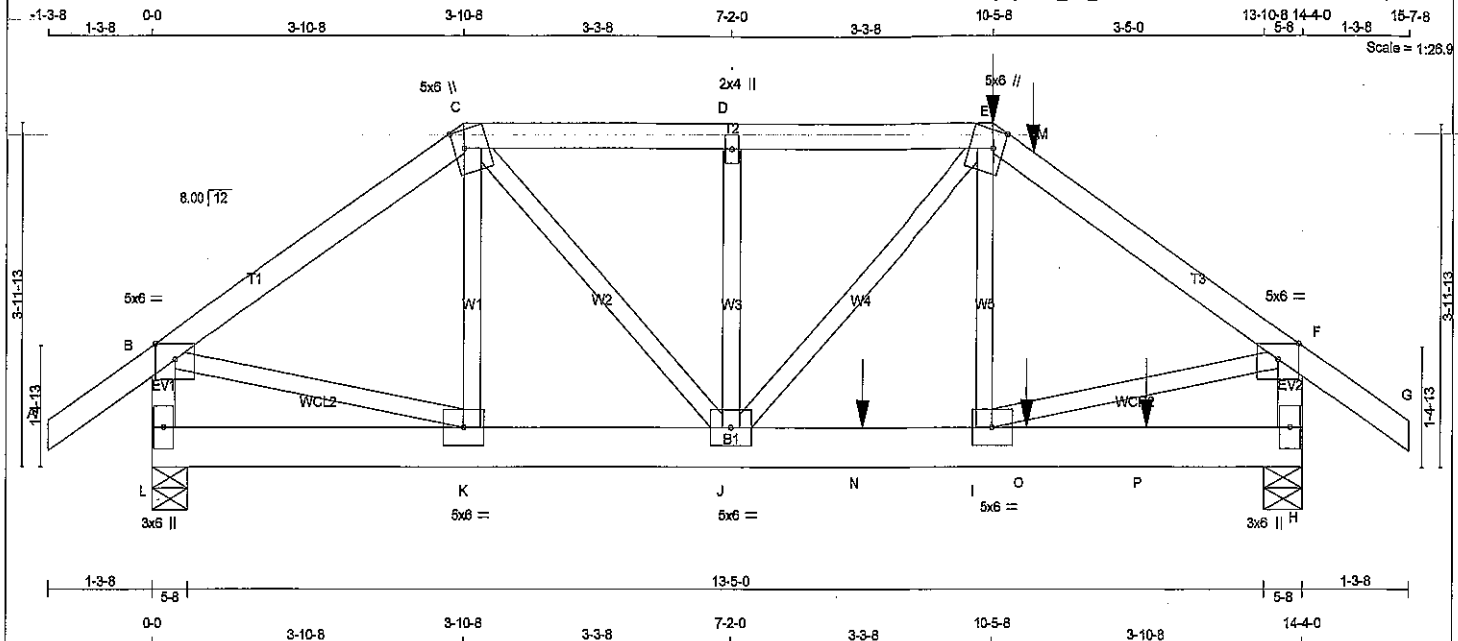
11318

S. KATSOULAKOS

PROVINCE OF ONTARIO

DWB NO. YAM 3337-18

STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 71 lb

LUMBER

N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR
A - C	2x4	DRY No.2	SPF
C - E	2x4	DRY No.2	SPF
E - G	2x4	DRY No.2	SPF
L - B	2x4	DRY No.2	SPF
H - F	2x4	DRY No.2	SPF
L - H	2x6	DRY No.2	SPF

ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
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DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWV-p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.50	1.50
D	TMW+h	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.50	1.50
F	TMWV-p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	6.0		
I	BMWV-t	MT20	5.0	6.0		
J	BMWVW-t	MT20	5.0	6.0		
K	BMWV-t	MT20	5.0	6.0		
L	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 193.4 lbs FACTORED DOWN AT 10-5-8, AND 62.0 lbs FACTORED DOWN AT 10-10-12 ON TOP CHORD, AND 587.8 lbs FACTORED DOWN AT 8-9-8, AND 33.0 lbs FACTORED DOWN AT 10-10-12, AND 41.0 lbs FACTORED DOWN AT 12-4-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
L	1220	0	1220	0	0	5-8	5-8
H	1498	0	1498	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	972	587 / 0	190 / 0	0 / 0	0 / 0	195 / 0	0 / 0
H	1190	723 / 0	230 / 0	0 / 0	0 / 0	238 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.95 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LOAD LC1	MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO			FR-TO				
A-B	0/32	-84.3	-84.3	0.12 (1)	10.00	K-C	-171/69	0.04 (1)	
B-C	-1158/0	-84.3	-84.3	0.27 (1)	5.56	C-J	0/1727	0.18 (1)	
C-D	-1448/0	-84.3	-84.3	0.17 (1)	5.22	J-D	-321/0	0.08 (1)	
D-E	-1448/0	-84.3	-84.3	0.17 (1)	5.22	J-E	0/250	0.06 (1)	
E-M	-1521/0	-84.3	-84.3	0.31 (1)	4.95	I-E	0/257	0.08 (2)	
M-F	-1521/0	-84.3	-84.3	0.31 (1)	4.95	B-K	0/892	0.25 (1)	
F-G	0/32	-84.3	-84.3	0.12 (1)	10.00	I-F	0/1314	0.33 (1)	
L-B	-1165/0	0.0	0.0	0.13 (1)	7.35				
H-F	-1480/0	0.0	0.0	0.16 (1)	6.73				
L-K	0/0	-28.0	-28.0	0.06 (2)	10.00				
K-J	0/957	-28.0	-28.0	0.20 (1)	10.00				
J-N	0/1279	-28.0	-28.0	0.45 (1)	10.00				
N-I	0/1279	-28.0	-28.0	0.45 (1)	10.00				
I-O	0/0	-28.0	-28.0	0.11 (2)	10.00				
O-P	0/0	-28.0	-28.0	0.11 (2)	10.00				
P-H	0/0	-28.0	-28.0	0.11 (2)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	D/R	TYPE
E	10-5-8	-183	-183	—	FRONT	VERT	TOTAL
M	10-10-12	-82	-82	—	FRONT	VERT	TOTAL
N	8-9-8	-588	-588	—	FRONT	VERT	TOTAL
O	10-10-12	-19	-33	—	FRONT	VERT	TOTAL
P	12-4-12	-23	-41	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD =		46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= $L/360$ (0.48")
 CALCULATED VERT. DEFL.(LL) = $L/999$ (0.04")
 ALLOWABLE DEFL.(TL)= $L/360$ (0.48")
 CALCULATED VERT. DEFL.(TL) = $L/999$ (0.06")

CSk: TC=0.31/1.00 (E-F:1), BC=0.45/1.00 (I-J:1),
WB=0.33/1.00 (F-I:1), SSI=0.23/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1867	822	2284	1856

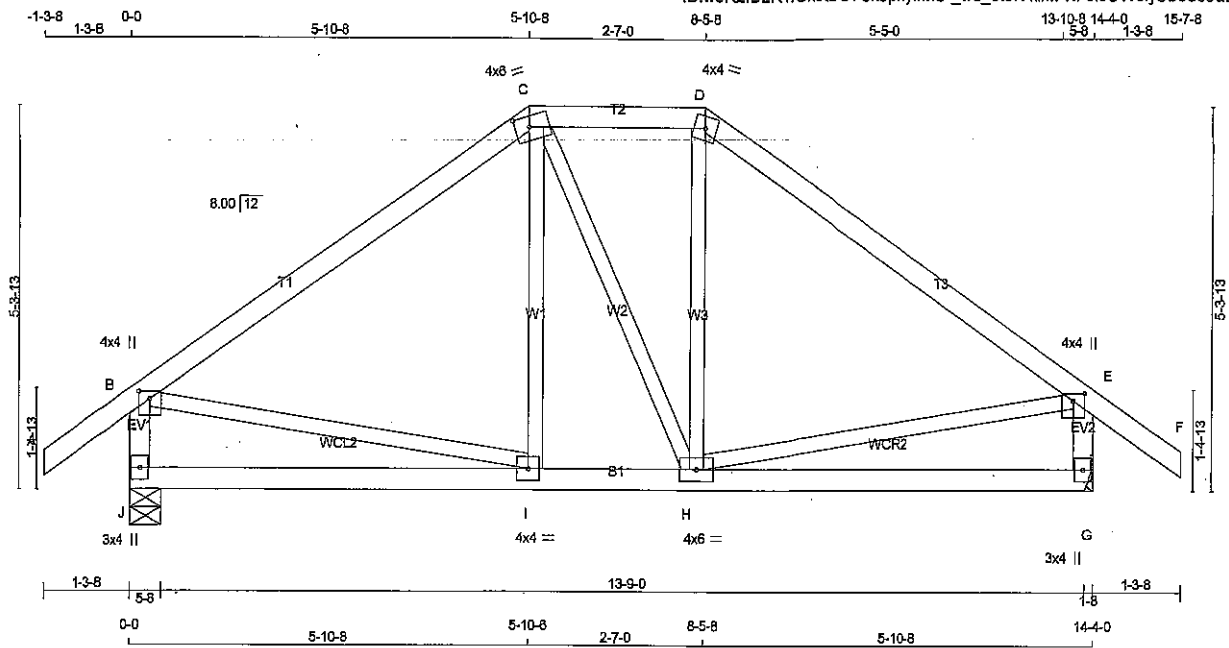
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (B) (INPUT = 0.90)
JSI METAL= 0.32 (I) (INPUT = 1.00)

DWG NO. TAM 2/21-18
STRUCTURAL
COMPONENT ONLY

JOB NAME 285862	TRUSS NAME T580	QUANTITY 1	PLY 1	JOB DESC. 43954	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Thu Apr 19 13:34:54 2018 Page 1 ID:w3K2IB2K4fGk9tDO7SkJphylfMC_wd_0t8N4ihlPrtF9i5UW5rjOb8ac9uDYVDphzP97?			



TOTAL WEIGHT = 63 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
J - B	2x4	DRY	No.2
G - E	2x4	DRY	No.2
J - G	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTWW-m	MT20	4.0	6.0	1.75	2.50
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	4.0	1.25	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
J	920	0	920	0	5-8	5-8
G	920	0	920	0	HANGER BY OTHERS	MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
J	739	437 / 0	151 / 0	0 / 0	0 / 0	152 / 0	0 / 0
G	739	437 / 0	151 / 0	0 / 0	0 / 0	152 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CS (I.C)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS (I.C)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 32	-84.3 -84.3	0.11 (1)	10.00	I-C	0 / 169	0.04 (3)
B-C	-588 / 0	-84.3 -84.3	0.38 (1)	6.25	C-H	0 / 5	0.00 (3)
C-D	-574 / 0	-84.3 -84.3	0.08 (1)	6.25	H-D	0 / 173	0.04 (3)
D-E	-689 / 0	-84.3 -84.3	0.38 (1)	6.25	B-I	0 / 581	0.13 (1)
E-F	0 / 32	-84.3 -84.3	0.11 (1)	10.00	H-E	0 / 582	0.13 (1)
J-B	-849 / 0	0.0	0.0	0.09 (1)			
G-E	-849 / 0	0.0	0.0	0.09 (1)			
J-I	0 / 0	-28.0 -28.0	0.21 (2)	10.00			
I-H	0 / 573	-28.0 -28.0	0.27 (2)	10.00			
H-G	0 / 0	-28.0 -28.0	0.21 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 3.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.48")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL) = L/360 (0.48")
CALCULATED VERT. DEFL.(TL) = L/999 (0.10")

CS: TC=0.38/1.00 (D-E:1), BC=0.27/1.00 (H-I:2), WB=0.13/1.00 (E-I:1), SS=0.16/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1887
	822	2284	1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (H) (INPUT = 0.90)
JSI METAL= 0.21 (I) (INPUT = 1.00)



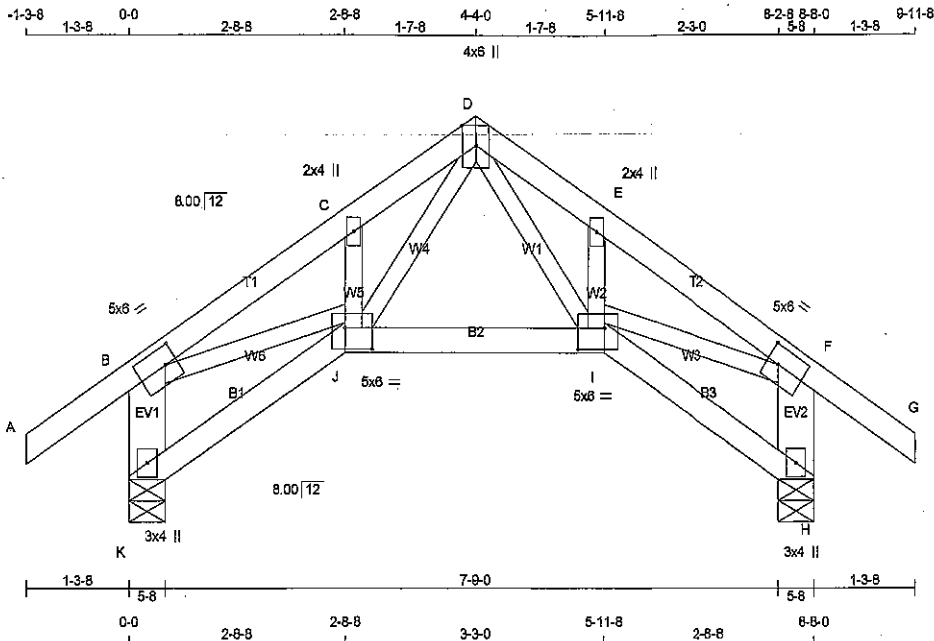
DWG NO. TAM21212-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
285862	T590	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 42 = 84 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
K - B	2x8	DRY	No.2
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
H - F	2x6	DRY	No.2
K - J	2x4	DRY	No.2
J - I	2x4	DRY	No.2
I - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	1.75
C	TMVW-w	MT20	2.0	4.0		
D	TTWW-p	MT20	4.0	6.0	Edge	
E	TMVW-w	MT20	2.0	4.0		
F	TMVW-t	MT20	5.0	6.0	2.50	1.75
H	BMV1-p	MT20	3.0	4.0		
I	BBWWW-p	MT20	5.0	6.0	3.00	4.00
J	BBWWW-p	MT20	5.0	6.0	3.00	4.00
K	BMV1-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
K	602	0	602	0	0	5-8	5-8	5-8	5-8
H	602	0	602	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	478	292/0	91/0	0/0	0/0	95/0	0/0
H	478	292/0	91/0	0/0	0/0	95/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED				WEBS		MAX. FACTORED			
MEMB.	FORCE (LBS)	VERT. LOAD	LC1	MAX	MAX.	MEMB.	FORCE (LBS)	MAX	MAX.	MEMB.	FORCE (LBS)
FR-TO		FROM	TO	LENGTH	FR-TO						
K-B	-584/0	0.0	0.0	0.04 (1)	7.81	D-I	0/480	0.11 (1)			
A-B	0/32	-84.3	-84.3	0.11 (1)	10.00	I-E	-209/0	0.03 (1)			
B-C	-808/0	-84.3	-84.3	0.09 (1)	6.25	I-F	0/685	0.15 (1)			
C-D	-825/0	-84.3	-84.3	0.03 (1)	6.25	J-D	0/480	0.11 (1)			
D-E	-825/0	-84.3	-84.3	0.03 (1)	6.25	J-C	-209/0	0.03 (1)			
E-F	-808/0	-84.3	-84.3	0.09 (1)	6.25	B-J	0/685	0.15 (1)			
F-G	0/32	-84.3	-84.3	0.11 (1)	10.00						
H-F	-584/0	0.0	0.0	0.04 (1)	7.81						
K-J	0/0	-28.0	-28.0	0.06 (3)	10.00						
J-I	0/414	-28.0	-28.0	0.13 (2)	10.00						
I-H	0/0	-28.0	-28.0	0.06 (3)	10.00						

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 240 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.29")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.29")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.11/1.00 (F-G:1), BC=0.13/1.00 (I-J:2), WB=0.15/1.00 (B-J:1), SSI=0.08/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1655

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.23 (B) (INPUT = 1.00)



DWG NO. TAM2123-18
STRUCTURAL
COMPONENT ONLY

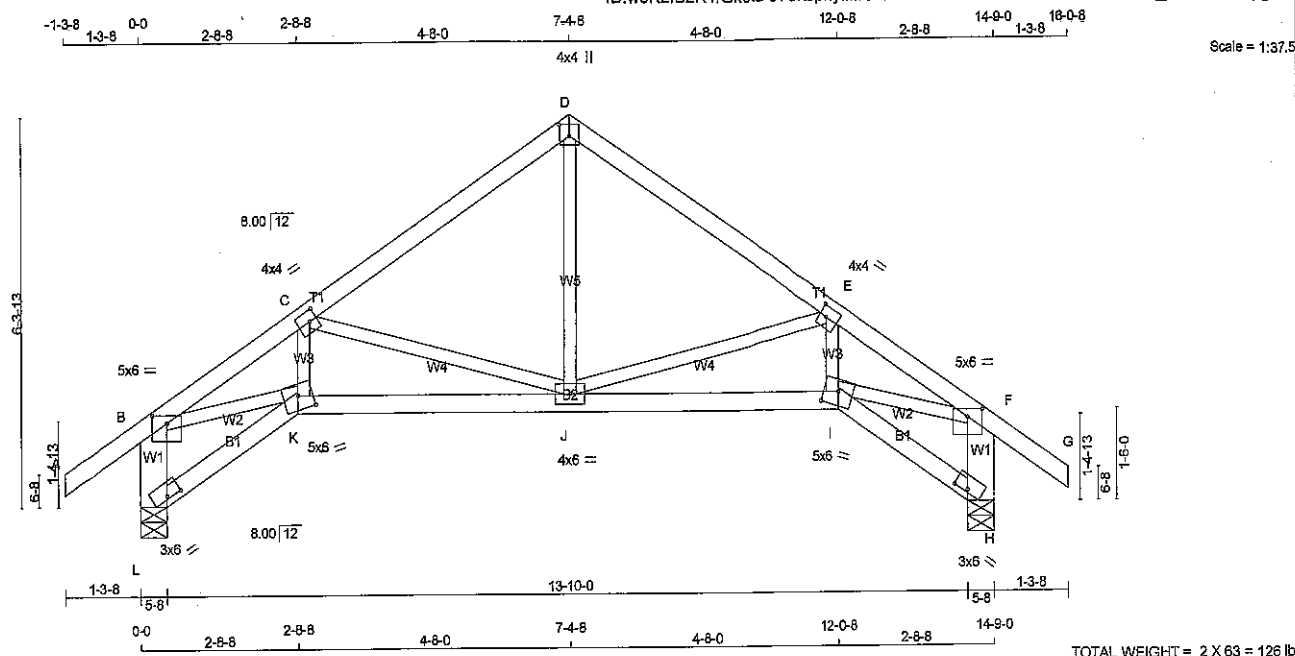


DWG NO. TAM 2124.18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285861	T610	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:w3K2IB2K4fGk9ID07SkJphyfMC-YCNMHYcXasxSZc5ucJ9cJBzuASLa_s?Xqoj1DKyg2IV



TOTAL WEIGHT = 2 X 63 = 126 lb

LUMBER	CHORDS	SIZE	DRY	LUMBER	DESCR.
N. L. G. A. RULES					
L - B	2x6	DRY	No.2	SPF	
A - D	2x4	DRY	No.2	SPF	
D - G	2x4	DRY	No.2	SPF	
H - F	2x6	DRY	No.2	SPF	
L - K	2x4	DRY	No.2	SPF	
K - I	2x4	DRY	No.2	SPF	
I - H	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)							
JT	TYPE	PLATES	W	LEN	Y	X	
B	TMVV-p	MT20	5.0	6.0	1.50	3.00	
C	TMVV-t	MT20	4.0	4.0	2.00	1.50	
D	TTW+p	MT20	4.0	4.0	2.25	2.00	
E	TMVV-t	MT20	4.0	4.0	2.00	1.50	
F	TMVV-p	MT20	5.0	6.0	1.50	3.00	
H	BVM1-I	MT20	3.0	6.0	0.50	3.00	
I	BBWW-m	MT20	5.0	6.0	2.75	3.00	
J	BMVVW-t	MT20	4.0	6.0			
K	BBWW-m	MT20	5.0	6.0	2.75	3.00	
L	BVM1-I	MT20	3.0	6.0	0.50	3.00	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX		
L	944	0	944	0	0	5-8	5-8		
H	944	0	944	0	0	5-8	5-8		

UNFACTORED REACTIONS

1ST CASE	MAX./MIN. COMPONENT REACTIONS								
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
L	758	448 / 0	155 / 0	0 / 0	0 / 0	158 / 0	0 / 0		
H	758	448 / 0	155 / 0	0 / 0	0 / 0	158 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.99 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX	MAX. UNBRAC	MEMB.	FORCE (LBS)	MAX. (LC)
FR-TO		FROM	TO	LENGTH	FR-TO		
L-B	-906 / 0	0.0	0.0	0.06 (1)	J-D	0 / 598	0.13 (1)
A-B	0 / 32	-84.3	-84.3	0.11 (1)	J-E	-732 / 0	0.28 (1)
B-C	-1676 / 0	-84.3	-84.3	0.16 (1)	I-E	0 / 320	0.07 (1)
C-D	-851 / 0	-84.3	-84.3	0.22 (1)	I-F	0 / 1434	0.32 (1)
D-E	-851 / 0	-84.3	-84.3	0.22 (1)	C-J	-732 / 0	0.28 (1)
E-F	-1676 / 0	-84.3	-84.3	0.16 (1)	K-C	0 / 320	0.07 (1)
F-G	0 / 32	-84.3	-84.3	0.11 (1)	B-K	0 / 1434	0.32 (1)
H-F	-906 / 0	0.0	0.0	0.06 (1)			
L-K	0 / 0	-28.0	-28.0	0.06 (2)			
K-J	0 / 1392	-28.0	-28.0	0.33 (2)			
J-I	0 / 1392	-28.0	-28.0	0.33 (2)			
I-H	0 / 0	-28.0	-28.0	0.06 (2)			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6	PSF
DL	= 3.0	PSF
BOT CH. LL	= 10.5	PSF
DL	= 7.0	PSF
TOTAL LOAD	= 46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.49")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.05")
ALLOWABLE DEFL.(TL) = $L/360$ (0.49")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.09")

CSI: TO=0.22 (C-D:1), BC=0.33 (I-J:2), WB=0.32 (F-I:1), SSI=0.14 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

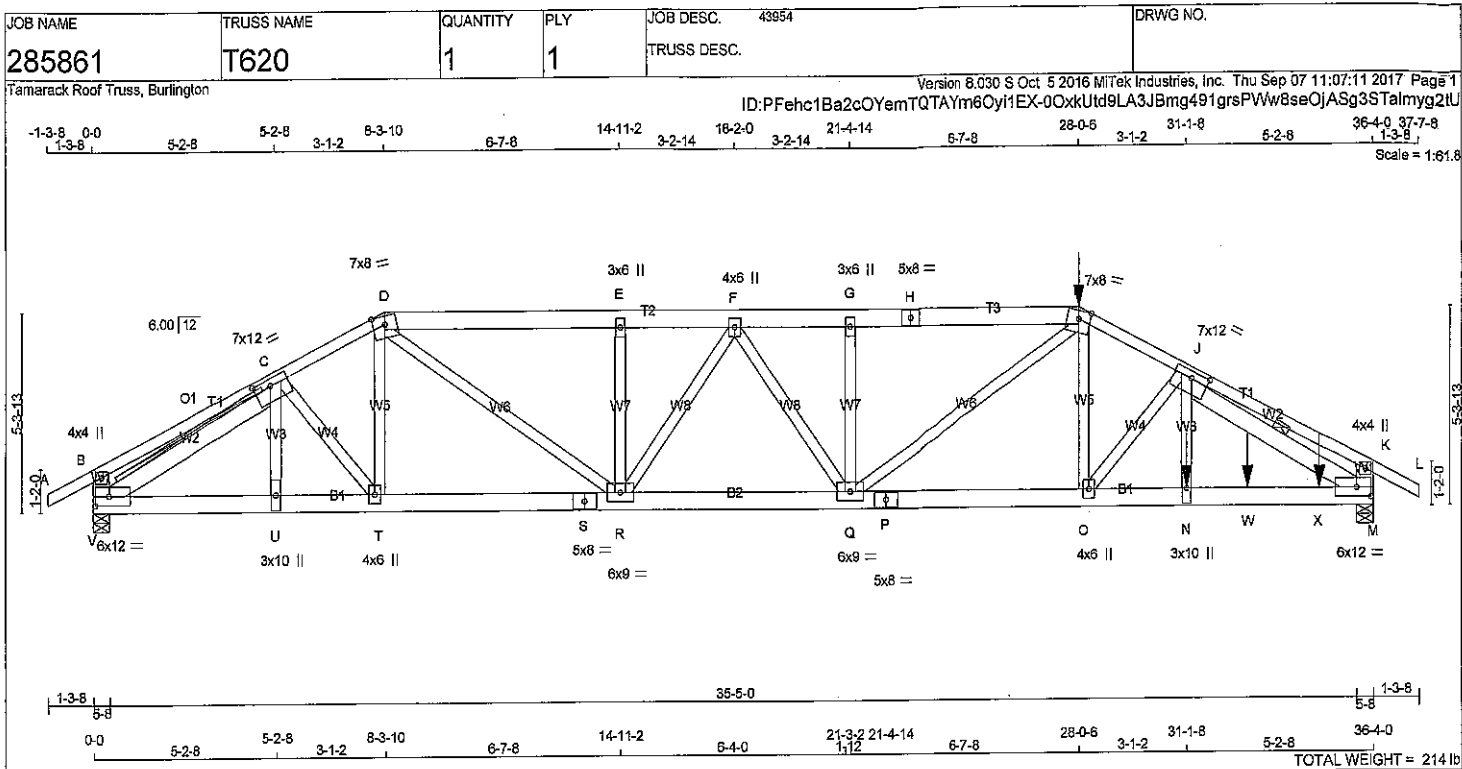
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (B) (INPUT = 0.90)
JSI METAL= 0.35 (I) (INPUT = 1.00)



DWG NO. TAW 45121 - 17
STRUCTURAL
COMPONENT ONLY



LUMBER

N.L.G.A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - H	2x6	DRY	No.2	SPF
H - I	2x6	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
V - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
V - S	2x8	DRY	2100F 1.8E	SPF
S - P	2x6	DRY	2100F 1.8E	SPF
P - M	2x6	DRY	2100F 1.8E	SPF
ALL WEBS	2x4	DRY	No.2	SPF
EXCEPT				
V - C	2x6	DRY	No.2	SPF
J - M	2x8	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMVWW-t	MT20	7.0	12.0	2.25	5.75
D	TTWW-m	MT20	7.0	8.0	Edge	
E	TMV+w	MT20	3.0	6.0		
F	TMVWW-t	MT20	4.0	6.0		
G	TMV+w	MT20	3.0	6.0		
H	TS-t	MT20	5.0	6.0		
I	TTWW-m	MT20	7.0	8.0	Edge	
J	TMVWW-t	MT20	7.0	12.0	2.25	5.75
K	TMV+p	MT20	4.0	4.0		
M	BMVW1-t	MT20	6.0	12.0	3.00	4.75
N	BMV+w	MT20	3.0	10.0		
O	BMVWW-t	MT20	4.0	6.0		
P	BS-t	MT20	5.0	6.0		
Q	BMVWW-t	MT20	6.0	9.0		
R	BMVWW-t	MT20	6.0	9.0		
S	BS-t	MT20	5.0	6.0		
T	BMVWW-t	MT20	4.0	6.0		
U	BMV+w	MT20	3.0	10.0		
V	BMVW1-t	MT20	6.0	12.0	3.00	4.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 499.5 lbs FACTORED DOWN AT 28-0.6 ON TOP CHORD, AND 2638.7 lbs FACTORED DOWN AT 31-1.8, AND 71.7 lbs FACTORED DOWN AT 32-10.4, AND 71.7 lbs FACTORED DOWN AT 34-10.4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
V	2652	0	2652	0	5-8	5-8
M	4876	0	4876	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE		MAX./MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	2145	1243 / 0	454 / 0	0 / 0	0 / 0	448 / 0	0 / 0
M	3952	2277 / 0	844 / 0	0 / 0	0 / 0	830 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.01 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BREAK(S) AT 1/2 LENGTH OF J-M.
2x4 DRY SPF No.2 T-BRACE AT C-V

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)		
FR-TO		FROM	TO		FR-TO				
A-B	0 / 28	-84.3	-84.3	0.12 (1)	10.00	U-C	0 / 168	0.03 (3)	
B-C	0 / 16	-84.3	-84.3	0.30 (1)	10.00	C-T	0 / 72	0.01 (2)	
C-D	-3967 / 0	-84.3	-84.3	0.52 (1)	3.00	T-D	0 / 239	0.04 (2)	
D-E	-5279 / 0	-84.3	-84.3	0.48 (1)	3.41	D-R	0 / 2168	0.38 (1)	
E-F	-5279 / 0	-84.3	-84.3	0.37 (1)	3.49	R-E	-579 / 0	0.16 (1)	
F-G	-5951 / 0	-84.3	-84.3	0.44 (1)	3.22	Q-G	-595 / 0	0.16 (1)	
G-H	-5950 / 0	-84.3	-84.3	0.55 (1)	3.16	O-I	0 / 509	0.09 (1)	
H-I	-5950 / 0	-84.3	-84.3	0.55 (1)	3.16	O-I	0 / 1847	0.29 (1)	
I-J	-8245 / 0	-84.3	-84.3	0.79 (1)	2.01	O-J	-1821 / 0	0.43 (1)	
J-K	-1 / 13	-84.3	-84.3	0.32 (1)	10.00	N-J	0 / 2813	0.46 (1)	
K-L	0 / 28	-84.3	-84.3	0.12 (1)	10.00	V-C	-4164 / 0	0.64 (1)	
V-B	-297 / 0	0.0	0.0	0.02 (1)	7.81	J-M	-8092 / 0	0.95 (1)	
M-K	-305 / 0	0.0	0.0	0.02 (1)	7.81	R-F	-648 / 0	0.26 (1)	
						F-O	0 / 384	0.40 (1)	
V-U	0 / 3494	-28.0	-28.0	0.24 (1)	10.00				
U-T	0 / 3494	-28.0	-28.0	0.23 (1)	10.00				
T-S	0 / 3534	-28.0	-28.0	0.23 (1)	10.00				
S-R	0 / 3534	-28.0	-28.0	0.23 (1)	10.00				
R-Q	0 / 5636	-28.0	-28.0	0.34 (1)	10.00				
Q-P	0 / 5542	-28.0	-28.0	0.33 (1)	10.00				
P-O	0 / 5542	-28.0	-28.0	0.33 (1)	10.00				
O-N	0 / 6787	-28.0	-28.0	0.46 (1)	10.00				
N-W	0 / 6789	-28.0	-28.0	0.49 (1)	10.00				
W-X	0 / 6789	-28.0	-28.0	0.49 (1)	10.00				
X-M	0 / 6789	-28.0	-28.0	0.49 (1)	10.00				

LICENSED PROFESSIONAL

9772

S. KATSOULAK

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.
I	28-0.6	-45	-50		FRONT	VERT
I	28-0.6	-455	-455		FRONT	VERT

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 3.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.21")
CALCULATED VERT. DEFL.(LL) = L/999 (0.28")
ALLOWABLE DEFL.(TL) = L/360 (1.21")
CALCULATED VERT. DEFL.(TL) = L/987 (0.44")

CSI: TC=0.79 (I-J); BC=0.48 (M-N); WB=0.95 (J-M); SSI=0.23 (G-I);

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1657 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (M) (INPUT = 0.90)
JSI METAL= 0.98 (C) (INPUT = 1.00)

DRWG NO. YAM 45122-17
STRUCTURAL
COMPONENT ONLY



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
285861	T620	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 6.030 8 Oct 5 2016 Mittek Industries, Inc. Thu Sep 07 11:07:11 2017 Page 2

ID:PFehc1Ba2cOYemTQTAYm6Oyi1EX-0OxkUtd9LA3JBmg491grsPWw8seOjASg3STalmyq2tU

FACTORED CONCENTRATED LOADS (LBS)							
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
N	31-1-8	-2639	-2639	---	FRONT	VERT	TOTAL
W	32-10-4	-41	-72	---	FRONT	VERT	TOTAL
X	34-10-4	-41	-72	---	FRONT	VERT	TOTAL



DWG NO. TAM 45122-17
 STRUCTURAL
 COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
285861	T620Z	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.030 8 Oct 5 2016 MiTek Industries, Inc. Thu Sep 07 11:07:11 2017. Page 2

ID:PFehc1Ba2cOYemTQTAYm8Oyi1EX-0OxkUtd9LA3JBmg491grsPVWw5seMIAQg3STalmvg2tU

FACTORED CONCENTRATED LOADS (LBS)						
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.
N	31-1-8	-2661	-2661	—	BACK	VERT
W	32-10-4	-41	-72	—	BACK	VERT
X	34-10-4	-41	-72	—	BACK	VERT
						TOTAL
						TOTAL
						TOTAL



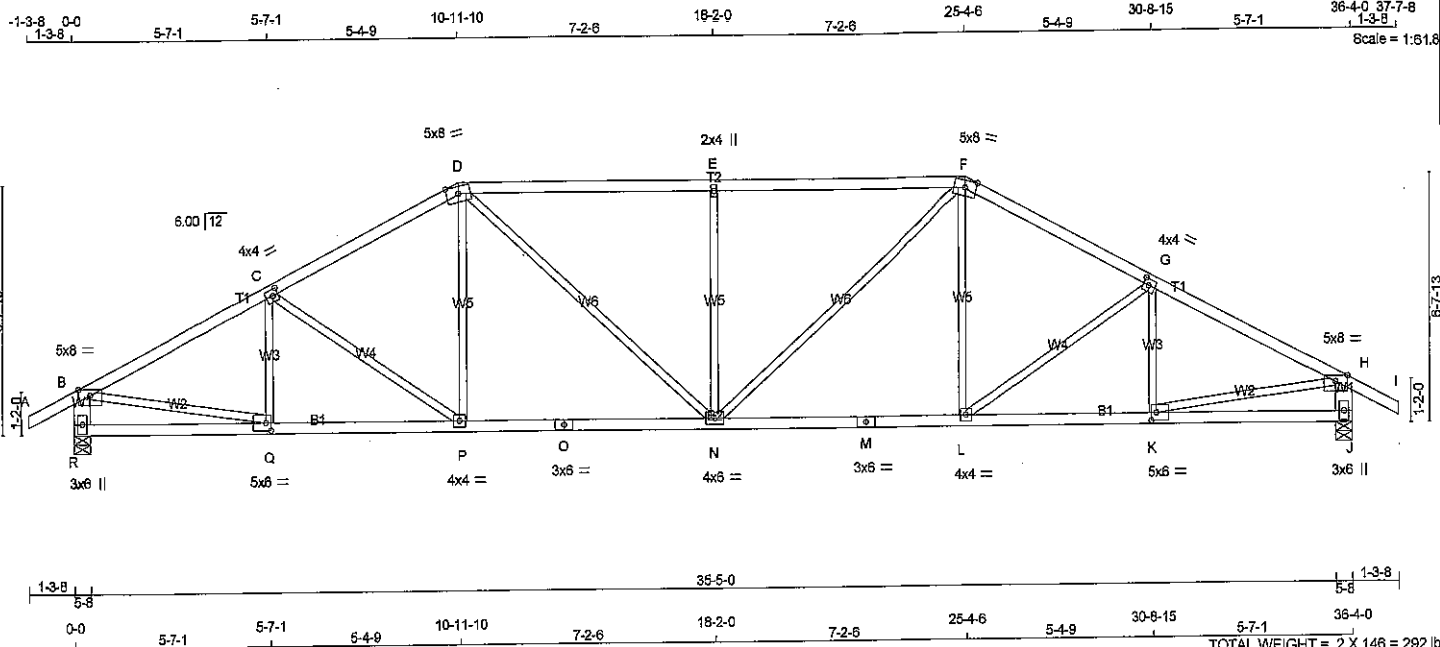
DWONG, YAM 45723-17
 STRUCTURAL
 COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
285861	T630	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.030 S Oct. 5 2016 Mitek Industries, Inc. Thu Sep 07 11:07:11 2017 Page 1

ID:w3K2IB2K4fGk9ID07SkJphylfMC-0OxkUtd9LA3JBmg491grsPVWwHscojFZg3Stalmyg2IU



LUMBER	SIZE	DRY	LUMBER	DESCR.
N. L. G. A. RULES				
CHORDS				
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	5.0	8.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	8.0	Edge	
E	TMWW-w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	8.0	Edge	
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
H	TMWW-p	MT20	5.0	8.0	Edge	
J	BMV1+p	MT20	3.0	6.0		
K	BMWW-t	MT20	5.0	8.0	2.50	1.75
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWW-t	MT20	4.0	8.0		
O	BS-t	MT20	3.0	6.0		
P	BMWW-t	MT20	4.0	4.0		
Q	BMWW-t	MT20	5.0	6.0	2.50	1.75
R	BMV1+p	MT20	3.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQD
JT	VERT	DOWN	BRG	IN-SX
R	2154	0	5-8	5-8
J	2154	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1752	999 / 0	382 / 0	0 / 0	0 / 0	371 / 0	0 / 0
J	1752	999 / 0	382 / 0	0 / 0	0 / 0	371 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.22 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)
FR-TO							
A-B	0 / 26	-84.3	-84.3	0.11 (1)	10.00	Q-C	-263 / 80
B-C	-3033 / 0	-84.3	-84.3	0.48 (1)	3.63	C-P	-317 / 0
C-D	-2787 / 0	-84.3	-84.3	0.44 (1)	3.80	P-D	0 / 422
D-E	-3013 / 0	-84.3	-84.3	0.78 (1)	3.22	D-N	0 / 718
E-F	-3013 / 0	-84.3	-84.3	0.78 (1)	3.22	N-E	-746 / 0
F-G	-2787 / 0	-84.3	-84.3	0.44 (1)	3.80	N-F	0 / 718
G-H	-3033 / 0	-84.3	-84.3	0.48 (1)	3.63	L-F	0 / 422
H-I	0 / 26	-84.3	-84.3	0.11 (1)	10.00	L-G	-317 / 0
R-B	-2085 / 0	0.0	0.0	0.13 (1)	7.03	K-G	-263 / 80
J-H	-2085 / 0	0.0	0.0	0.13 (1)	7.03	B-Q	0 / 2765
R-Q	0 / 0	-28.0	-28.0	0.18 (2)	10.00	K-H	0 / 2765
Q-P	0 / 2733	-28.0	-28.0	0.57 (1)	10.00		
P-O	0 / 2475	-28.0	-28.0	0.59 (2)	10.00		
O-N	0 / 2475	-28.0	-28.0	0.59 (2)	10.00		
N-M	0 / 2475	-28.0	-28.0	0.59 (2)	10.00		
M-L	0 / 2475	-28.0	-28.0	0.59 (2)	10.00		
L-K	0 / 2733	-28.0	-28.0	0.57 (1)	10.00		
K-J	0 / 0	-28.0	-28.0	0.18 (2)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 25.8 PSF
	DL	= 3.0 PSF
BOT CH.	LL	= 10.5 PSF
	DL	= 7.0 PSF
TOTAL LOAD		= 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (1.21")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.22")
ALLOWABLE DEFL.(TL) = $L/360$ (1.21")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.38")

CSI: TC=0.78 (D-E:1), BC=0.59 (N-P:2), WB=0.62 (B-Q:1), SSI=0.28 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1567 822	2284 1656

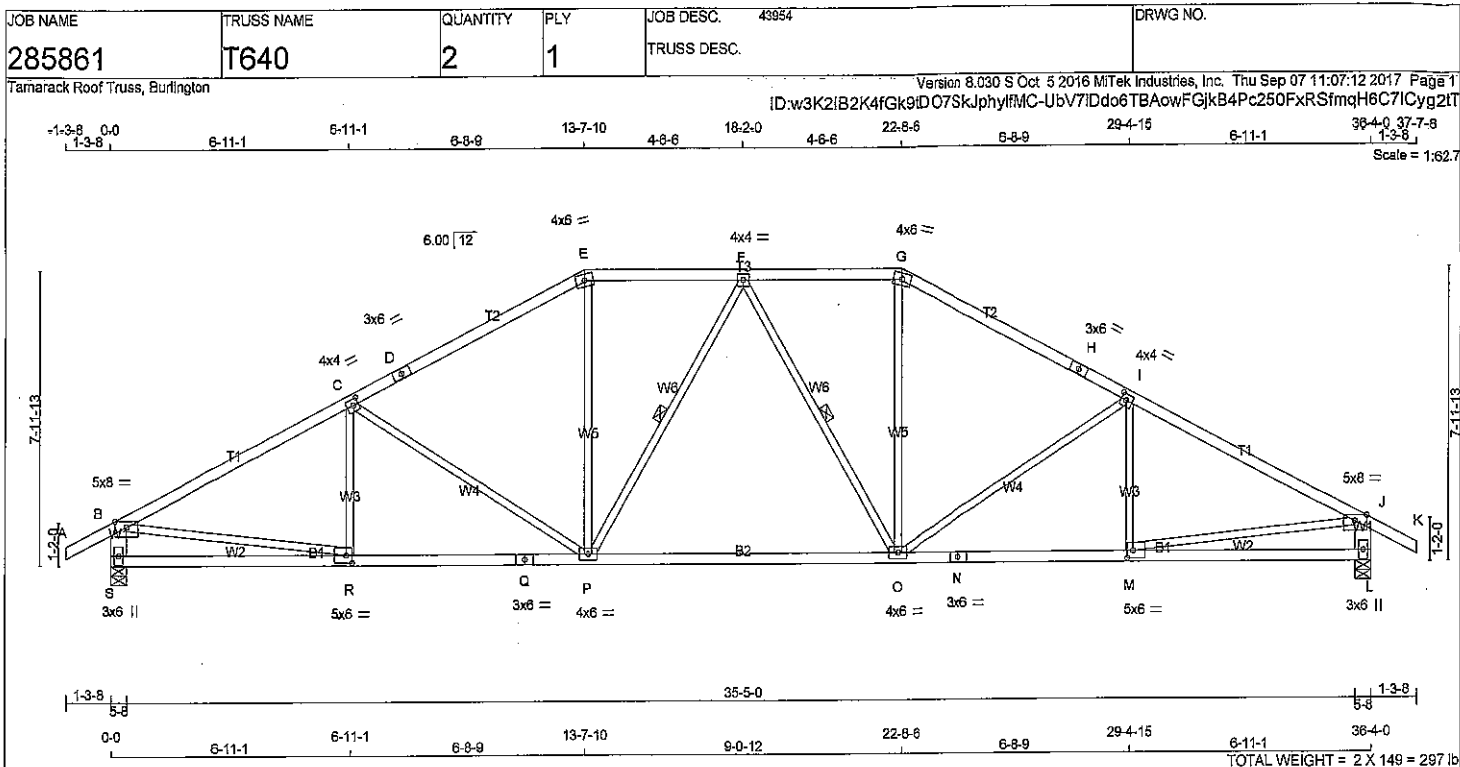
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (N) (INPUT = 0.90)
JSI METAL= 0.76 (M) (INPUT = 1.00)



DRWG NO. TAM45124-17
STRUCTURAL
COMPONENT ONLY



LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	No.2	SPF
A - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
G - H	2x4	DRY	No.2	SPF	
H - K	2x4	DRY	No.2	SPF	
S - B	2x6	DRY	No.2	SPF	
L - J	2x6	DRY	No.2	SPF	
S - Q	2x4	DRY	No.2	SPF	
Q - N	2x4	DRY	No.2	SPF	
N - L	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMVW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	6.0		
E	TTW-m	MT20	4.0	6.0		
F	TMVW-t	MT20	4.0	4.0		
G	TTW-m	MT20	4.0	6.0		
H	TS-t	MT20	3.0	6.0		
I	TMVW-t	MT20	4.0	4.0	2.00	1.75
J	TMVW-p	MT20	5.0	8.0	Edge	
L	BMV1+p	MT20	3.0	6.0		
M	BMVW-t	MT20	5.0	6.0	2.50	2.00
N	BS-t	MT20	3.0	6.0		
O	BMVW-t	MT20	4.0	6.0		
P	BMVW-t	MT20	4.0	6.0		
Q	BS-t	MT20	3.0	6.0		
R	BMVW-t	MT20	5.0	6.0	2.50	2.00
S	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD IN-SX
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
S	2154	0	2154	0	5-8
L	2154	0	2154	0	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. SNOW	MIN. LIVE	PERM. LIVE	WIND	DEAD	SOIL
S	1752	999 / 0	382 / 0	0 / 0	0 / 0	371 / 0	0 / 0
L	1752	999 / 0	382 / 0	0 / 0	0 / 0	371 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.28 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-P, F-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX. FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRAC	CS1 (LC)	MEMB.	FORCE (LBS)	MAX. UNBRAC	CS1 (LC)
FR-TO						FR-TO			
A-B	0 / 26	-84.3	-84.3	0.11 (1)	10.00	R-C	-146 / 185	0.05 (1)	
B-C	-3074 / 0	-84.3	-84.3	0.78 (1)	3.28	C-P	-593 / 0	0.82 (1)	
C-D	-2576 / 0	-84.3	-84.3	0.67 (1)	3.64	P-E	0 / 775	0.17 (1)	
D-E	-2576 / 0	-84.3	-84.3	0.67 (1)	3.64	F-F	-265 / 0	0.15 (1)	
E-F	-2285 / 0	-84.3	-84.3	0.27 (1)	4.29	F-O	-265 / 0	0.15 (1)	
F-G	-2285 / 0	-84.3	-84.3	0.27 (1)	4.29	O-G	0 / 775	0.17 (1)	
G-H	-2576 / 0	-84.3	-84.3	0.67 (1)	3.64	O-I	-593 / 0	0.82 (1)	
H-I	-2576 / 0	-84.3	-84.3	0.67 (1)	3.64	M-I	-146 / 185	0.05 (1)	
I-J	-3074 / 0	-84.3	-84.3	0.78 (1)	3.28	B-R	0 / 2797	0.63 (1)	
J-K	0 / 26	-84.3	-84.3	0.11 (1)	10.00	M-J	0 / 2797	0.63 (1)	
S-B	-2071 / 0	0.0	0.0	0.13 (1)	7.05				
L-J	-2071 / 0	0.0	0.0	0.13 (1)	7.05				
S-R	0 / 0	-28.0	-28.0	0.28 (3)	10.00				
R-Q	0 / 2776	-28.0	-28.0	0.69 (2)	10.00				
Q-P	0 / 2776	-28.0	-28.0	0.69 (2)	10.00				
P-O	0 / 2418	-28.0	-28.0	0.65 (2)	10.00				
O-N	0 / 2776	-28.0	-28.0	0.69 (2)	10.00				
N-M	0 / 2776	-28.0	-28.0	0.69 (2)	10.00				
M-L	0 / 0	-28.0	-28.0	0.28 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN/GC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = 1/360 (1.21")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.27")
ALLOWABLE DEFL.(TL) = 1/360 (1.21")
CALCULATED VERT. DEFL.(TL) = 1/985 (0.44")

CS1: TC=0.78 (B-C:1), BC=0.69 (P-R:2), WB=0.82 (C-P:1), SSI=0.25 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.71 (N) (INPUT = 1.00)



DWG NO. TAM 45/25-17
STRUCTURAL
COMPONENT ONLY

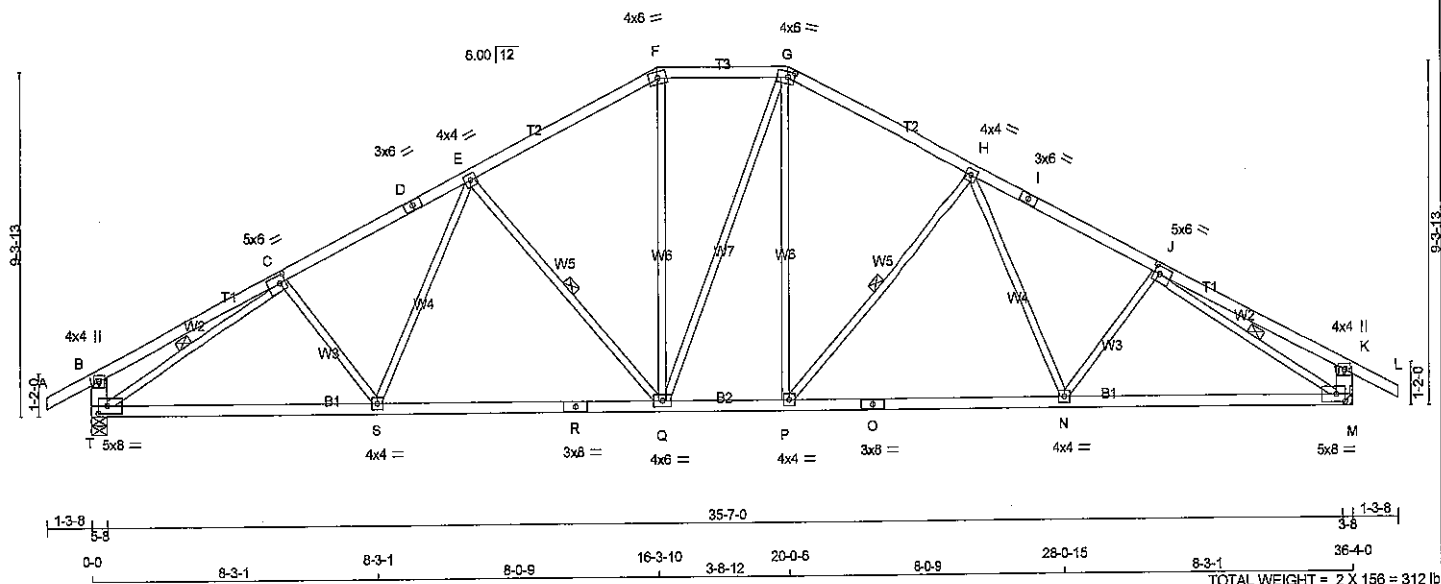
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285861	T650	2	1	43954	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 Mitek Industries, Inc. Thu Sep 07 11:07:12 2017 Page 1

ID:w3K2IB2K4fGk9tD07SkJphyfMC-Ubv7IDdo8TBAowFGjKb4Pc2BfvtSdTh6C7ICyg2IT

-1-3-8 0-0 5-6-14 5-6-14 5-4-6 10-11-4 5-4-6 16-3-10 3-8-12 20-0-6 5-4-6 25-4-12 5-4-6 30-9-2 5-6-14 36-4-0 37-7-8 1-3-8
Scale = 1:62.7



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - L	2x4	DRY	No.2
T - B	2x6	DRY	No.2
M - K	2x6	DRY	No.2
T - R	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - M	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			SPF
DRY: SEASONED LUMBER.			

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	4.0	4.0
C	TMWW-t	MT20	5.0	8.0 2.50 2.00
D	TS-t	MT20	3.0	6.0
E	TMWW-t	MT20	4.0	4.0
F	TTW-m	MT20	4.0	6.0
G	TTWW-m	MT20	4.0	8.0 1.75 2.25
H	TMWW-t	MT20	4.0	4.0
I	TS-t	MT20	3.0	6.0
J	TMWW-t	MT20	5.0	8.0 2.50 2.00
K	TMV+p	MT20	4.0	4.0
M	BMVW-t	MT20	5.0	8.0 2.50 3.00
N, P, S	BMVW-t	MT20	4.0	4.0
O	BS-t	MT20	3.0	8.0
Q	BMVW-t	MT20	4.0	6.0
R	BS-t	MT20	3.0	8.0
T	BMVW-t	MT20	5.0	8.0 2.50 3.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECORD BRG
JT	VERT	SNOW	DOWN	IN-SX	IN-SX
T	2154	0	2154	0	5-8
M	2154	0	2154	0	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
T	1752	999 / 0	382 / 0	0 / 0	371 / 0
M	1752	999 / 0	382 / 0	0 / 0	371 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.76 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-Q, H-P, C-T, J-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED			
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 26	-84.3 -84.3	0.11 (1)	10.00	C-S	-72 / 148 0.03 (3)	
B-C	0 / 18	-84.3 -84.3	0.30 (1)	10.00	S-E	0 / 424 0.10 (2)	
C-D	-2939 / 0	-84.3 -84.3	0.41 (1)	3.76	E-Q	-701 / 0 0.34 (1)	
D-E	-2939 / 0	-84.3 -84.3	0.41 (1)	3.76	Q-F	0 / 683 0.15 (1)	
E-F	-2298 / 0	-84.3 -84.3	0.37 (1)	4.20	C-G	0 / 4 0.00 (1)	
F-G	-2044 / 0	-84.3 -84.3	0.20 (1)	4.57	F-G	0 / 679 0.15 (1)	
G-H	-2298 / 0	-84.3 -84.3	0.37 (1)	4.20	F-H	-703 / 0 0.34 (1)	
H-I	-2940 / 0	-84.3 -84.3	0.41 (1)	3.76	H-N	0 / 426 0.10 (2)	
I-J	-2940 / 0	-84.3 -84.3	0.41 (1)	3.76	N-J	-71 / 148 0.03 (3)	
J-K	0 / 18	-84.3 -84.3	0.30 (1)	10.00	T-C	-3199 / 0 0.90 (1)	
K-L	0 / 26	-84.3 -84.3	0.11 (1)	10.00	J-M	-3200 / 0 0.90 (1)	
T-B	-305 / 0	0.0 0.0	0.02 (1)	7.81			
M-K	-305 / 0	0.0 0.0	0.02 (1)	7.81			
T-S	0 / 2674	-28.0 -28.0	0.79 (2)	10.00			
S-R	0 / 2494	-28.0 -28.0	0.77 (2)	10.00			
R-Q	0 / 2494	-28.0 -28.0	0.77 (2)	10.00			
Q-P	0 / 2042	-28.0 -28.0	0.48 (2)	10.00			
P-O	0 / 2494	-28.0 -28.0	0.78 (2)	10.00			
O-N	0 / 2494	-28.0 -28.0	0.78 (2)	10.00			
N-M	0 / 2674	-28.0 -28.0	0.79 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 3.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 48.1	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCG 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.21")
CALCULATED VERT. DEFL.(LL) = L/999 (0.22")
ALLOWABLE DEFL.(TL) = L/360 (1.21")
CALCULATED VERT. DEFL.(TL) = L/999 (0.38")

CSI: TC=0.41 (H-J:1), BC=0.79 (M-N:2), WB=0.90 (J-M:1), SSI=0.19 (M-N:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618 354 1867 822 2284 1856	

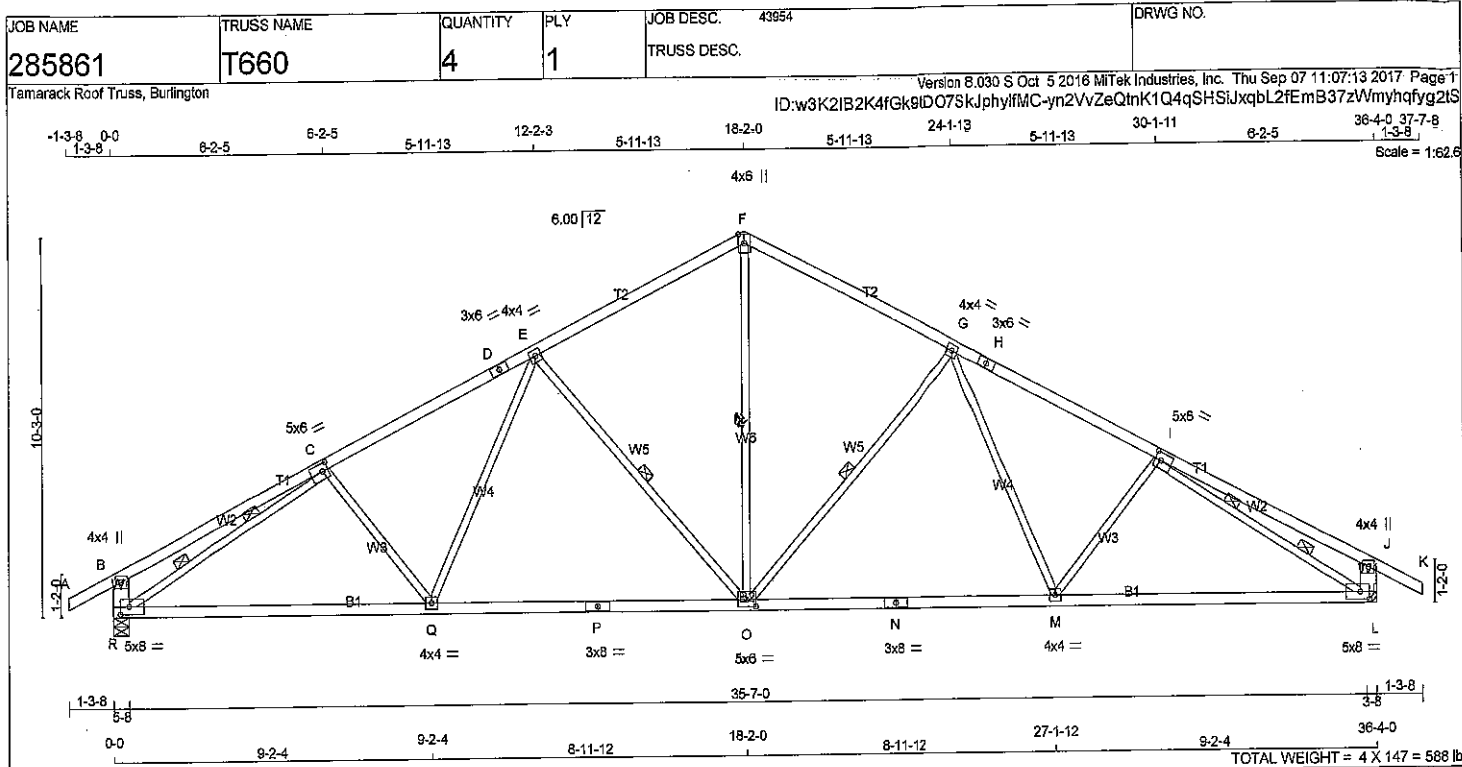
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (M) (INPUT = 0.90)
JSI METAL= 0.80 (O) (INPUT = 1.00)



DWG NO. TAM 45126-17
STRUCTURAL
COMPONENT ONLY



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
R - B	2x6	DRY	No.2
L - J	2x6	DRY	No.2
R - P	2x4	DRY	No.2
P - N	2x4	DRY	No.2
N - L	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.00
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTW+p	MT20	4.0	6.0	Edge	
G	TMWW-t	MT20	4.0	4.0		
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	5.0	6.0	2.50	2.00
J	TMV+p	MT20	4.0	4.0		
L	BMVW1-t	MT20	5.0	8.0	2.50	3.00
M	BMWW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	8.0		
O	BMVWW-t	MT20	5.0	6.0	2.25	3.00
P	BS-t	MT20	3.0	8.0		
Q	BMVW1-t	MT20	4.0	4.0		
R	BMVW1-t	MT20	5.0	6.0	2.50	3.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
R	2154	0	2154	0	5-8	5-8
L	2154	0	2154	0	5-8	5-8

HANGER BY OTHERS
MIN. SEAT SIZE: 3-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
R	1752	999 / 0	382 / 0	0 / 0	0 / 0	371 / 0	0 / 0
L	1752	999 / 0	382 / 0	0 / 0	0 / 0	371 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.70 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-O, E-O. **F-O**
2 LATERAL BRACE(S) AT 1/3 LENGTH OF C-R, L-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED			MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED		
		VERT. LOAD	LC1	MAX (PLF) CSI (LC)			UNBRAC LENGTH	FR-TO CSI (LC)	
FR-TO		FROM	TO						
A-B	0/26	-84.3	-84.3	0.11 (1)	10.00	O-F	0/1475	0.33 (1)	
B-C	0/21	-84.3	-84.3	0.39 (1)	10.00	O-G	-816/0	0.48 (1)	
C-D	-2896/0	-84.3	-84.3	0.50 (1)	3.70	G-M	0/509	0.11 (2)	
D-E	-2896/0	-84.3	-84.3	0.50 (1)	3.70	M-I	-163/124	0.07 (1)	
E-F	-2124/0	-84.3	-84.3	0.45 (1)	4.25	E-O	-816/0	0.48 (1)	
F-G	-2124/0	-84.3	-84.3	0.45 (1)	4.25	Q-E	0/509	0.11 (2)	
G-H	-2896/0	-84.3	-84.3	0.50 (1)	3.70	C-Q	-163/124	0.07 (1)	
H-I	-2896/0	-84.3	-84.3	0.50 (1)	3.70	R-C	-3197/0	1.00 (1)	
I-J	0/21	-84.3	-84.3	0.39 (1)	10.00	I-L	-3197/0	1.00 (1)	
J-K	0/26	-84.3	-84.3	0.11 (1)	10.00				
R-B	-325/0	0.0	0.0	0.02 (1)	7.81				
L-J	-325/0	0.0	0.0	0.02 (1)	7.81				
R-Q	0/2692	-28.0	-28.0	0.88 (2)	10.00				
Q-P	0/2413	-28.0	-28.0	0.85 (2)	10.00				
P-O	0/2413	-28.0	-28.0	0.85 (2)	10.00				
O-N	0/2413	-28.0	-28.0	0.85 (2)	10.00				
N-M	0/2413	-28.0	-28.0	0.85 (2)	10.00				
M-L	0/2692	-28.0	-28.0	0.88 (2)	10.00				

REGISTERED PROFESSIONAL

9777

S. KATSOULAKO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6	PSF
DL	= 3.0	PSF
BOT CH. LL	= 10.5	PSF
DL	= 7.0	PSF
TOTAL LOAD	= 45.1	PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.21")
CALCULATED VERT. DEFL.(LL) = L/999 (0.31")
ALLOWABLE DEFL.(TL) = L/360 (1.21")
CALCULATED VERT. DEFL.(TL) = L/863 (0.51")

CS1: TC=0.50 (G-I-1), BC=0.88 (Q-R-2), WB=1.00 (I-L-1), SS=0.21 (I-L-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

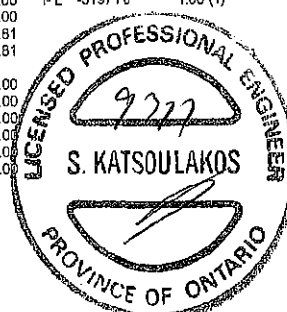
PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PL)
MT20	618	354
1667	822	2284
1666		

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (L) (INPUT = 0.90)

JSI METAL= 0.80 (N) (INPUT = 1.00)



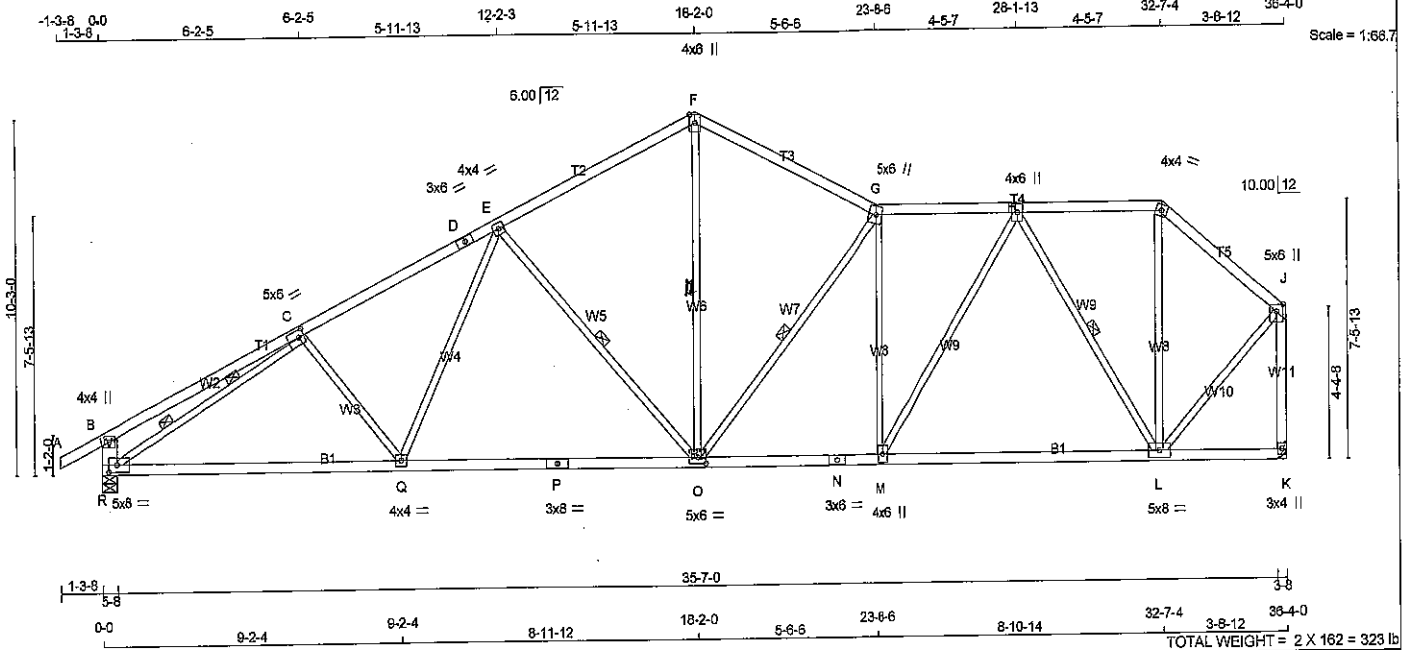
DWG NO. TAM 45127-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
285861	T670S	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.030 5 Oct 5 2016 MITek Industries, Inc. Thu Sep 07 11:07:13 2017, Page 1

ID:PFehc1Ba2cOYemTQTAYm6Oyi1EX-yn2VvZeQtnK1Q4qSHSijxqblZfDJB36zVmyhqfyg2tS



LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N - L, G, A, RULES				
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - J	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.00
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTW+p	MT20	4.0	6.0	Edge	
G	TTWW+m	MT20	5.0	6.0		
H	TMWW+t	MT20	4.0	6.0		
I	TTW-m	MT20	4.0	4.0		
J	TMWW+p	MT20	5.0	6.0	Edge	
K	BMV1+p	MT20	3.0	4.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMWW-t	MT20	4.0	6.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	5.0	6.0	2.25	3.00
P	BS-t	MT20	3.0	6.0		
Q	BMWW-t	MT20	4.0	4.0		
R	BMWW-t	MT20	5.0	6.0	2.50	3.00

Edges - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
R	2154	0	2154	0
K	2039	0	2039	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	PERM.LIVE	WIND	DEAD	SOIL
R	COMBINED	SNOW	LIVE	0/0	371/0	0/0
K	1674	999/0	382/0	0/0	383/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.70 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

- 1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-O, G-O, H-L.
- 2 LATERAL BRACE(S) AT 1/3 LENGTH OF C-R.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0/26	-84.3 -84.3 0.11 (1)	10.00	C-Q	-163/124	0.07 (1)	
B-C	0/21	-84.3 -84.3 0.39 (1)	10.00	Q-E	0/524	0.12 (2)	
C-D	-2698/0	-84.3 -84.3 0.50 (1)	3.70	E-O	-822/0	0.49 (1)	
D-E	-2698/0	-84.3 -84.3 0.50 (1)	3.70	O-F	0/1436	0.32 (1)	
E-F	-2118/0	-84.3 -84.3 0.44 (1)	4.25	F-G	-788/0	0.45 (1)	
F-G	-2095/0	-84.3 -84.3 0.41 (1)	4.30	G-H	-890/0	0.70 (1)	
G-H	-2345/0	-84.3 -84.3 0.28 (1)	4.24	H-I	0/1039	0.23 (1)	
H-I	-994/0	-84.3 -84.3 0.22 (1)	6.00	I-L	-1572/0	0.76 (1)	
I-J	-1288/0	-84.3 -84.3 0.17 (1)	5.51	L-I	0/523	0.12 (1)	
J-K	-325/0	0.0 0.0 0.02 (1)	7.81	R-C	-2200/0	1.00 (1)	
K-J	-2025/0	0.0 0.0 0.66 (1)	5.94	L-K	0/1039	0.23 (1)	
R-Q	0/2894	-28.0 -28.0 0.91 (2)	10.00				
Q-P	0/2411	-28.0 -28.0 0.88 (2)	10.00				
P-O	0/2411	-28.0 -28.0 0.88 (2)	10.00				
O-N	0/2355	-28.0 -28.0 0.59 (2)	10.00				
N-M	0/2355	-28.0 -28.0 0.59 (2)	10.00				
M-L	0/1807	-28.0 -28.0 0.57 (2)	10.00				
L-K	0/0	-28.0 -28.0 0.36 (3)	10.00				

REGISTERED PROFESSIONAL E

9777

S. KATSOULAKOS

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	25.6	PSF
DL	=	3.0	PSF
BOT CH. LL	=	10.5	PSF
DL	=	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN./C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.21")
CALCULATED VERT. DEFL.(LL) = L/999 (0.29")
ALLOWABLE DEFL.(TL) = L/360 (1.21")
CALCULATED VERT. DEFL.(TL) = L/807 (0.48")

CSI: TC=0.66 (J-K:1), BC=0.91 (Q-R:2), WB=1.00 (C-R:1), SSI=0.21 (Q-R:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

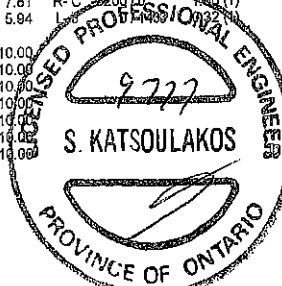
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354
	1887	822
	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

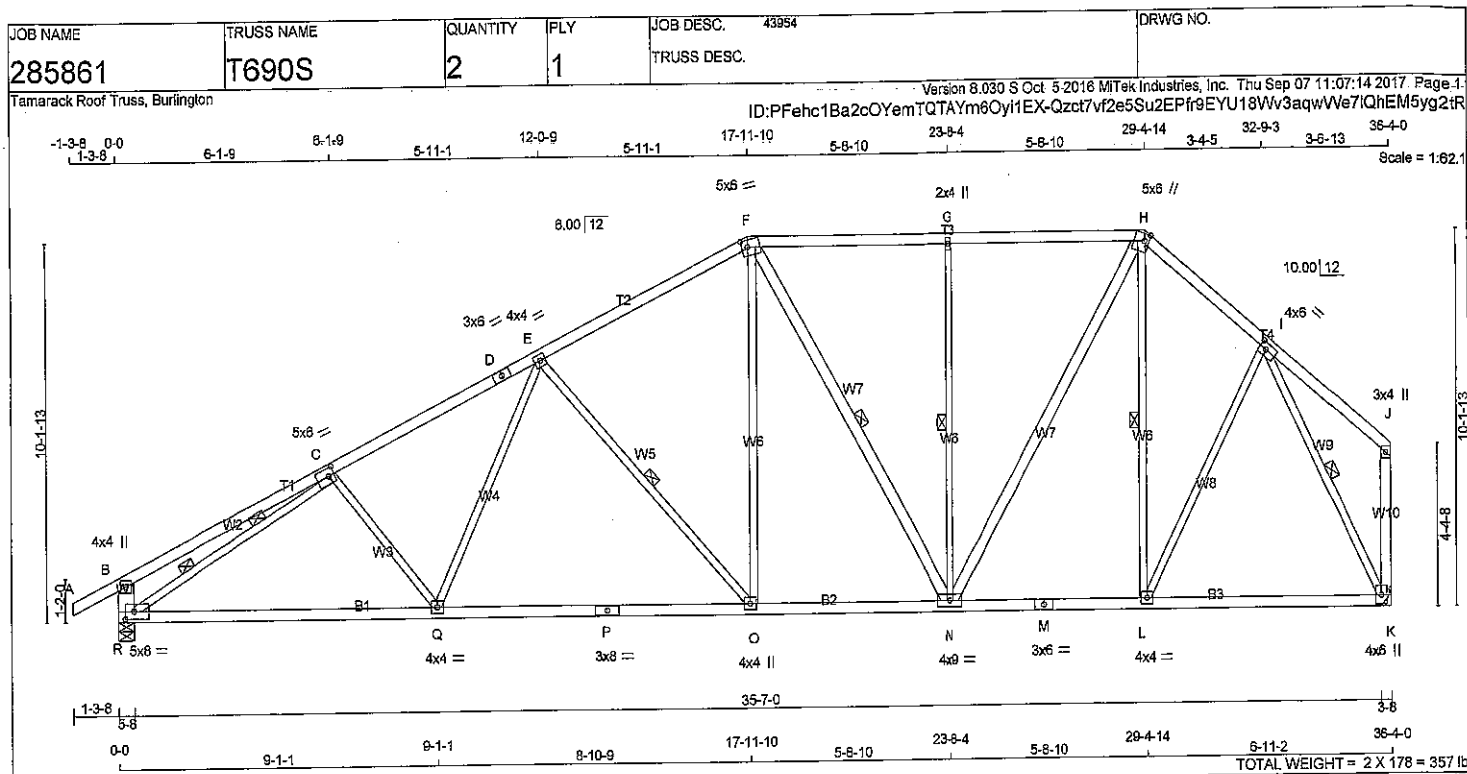
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (R) (INPUT = 0.90)
JSI METAL= 0.85 (P) (INPUT = 1.00)



DRWG NO. T670S12B17
STRUCTURAL
COMPONENT ONLY





LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - J	2x4	DRY	No.2
R - B	2x6	DRY	No.2
K - J	2x4	DRY	No.2
R - P	2x4	DRY	No.2
P - M	2x4	DRY	No.2
M - K	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2

EXCEPT

F - N 2x4 DRY No.2

N - H 2x4 DRY No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	5.0	8.0	2.50	2.00
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTWW-m	MT20	5.0	8.0	2.25	2.00
G	TMW+w	MT20	2.0	4.0		
H	TTWW+m	MT20	5.0	8.0	2.25	1.50
I	TMWW-t	MT20	4.0	6.0	2.00	2.25
J	TMV+p	MT20	3.0	4.0		
K	BMVW1+p	MT20	4.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMVW1-t	MT20	4.0	9.0		
O	BMVW1-t	MT20	4.0	4.0		
P	BS-t	MT20	3.0	8.0		
Q	BMVW1-t	MT20	4.0	4.0		
R	BMVW1-t	MT20	5.0	8.0	2.50	3.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
R	2154	0	2154	0	0	5-8	5-8
K	2039	0	2039	0	0	HANGER BY OTHERS	MIN. SEAT SIZE: 3-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1752	999 / 0	382 / 0	0 / 0	0 / 0	371 / 0	0 / 0
K	1674	930 / 0	382 / 0	0 / 0	0 / 0	363 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.71 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-O, F-N, G-N, H-L, I-K.
2 LATERAL BRACE(S) AT 1/3 LENGTH OF C-R.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO				FR-TO			
A-B	0 / 26	-84.3	-84.3 0.11 (1)	C-Q	-153 / 127	0.06 (1)	
B-C	0 / 20	-84.3	-84.3 0.38 (1)	Q-E	0 / 511	0.11 (2)	
C-D	-2903 / 0	-84.3	-84.3 0.50 (1)	E-O	-813 / 0	0.48 (1)	
D-E	-2903 / 0	-84.3	-84.3 0.50 (1)	O-F	0 / 821	0.18 (1)	
E-F	-2132 / 0	-84.3	-84.3 0.44 (1)	F-N	-301 / 0	0.21 (1)	
F-G	-1743 / 0	-84.3	-84.3 0.40 (1)	N-G	-588 / 0	0.42 (1)	
G-H	-1743 / 0	-84.3	-84.3 0.40 (1)	H-N	0 / 1128	0.18 (1)	
H-I	-1562 / 0	-84.3	-84.3 0.15 (1)	I-L	-284 / 36	0.20 (1)	
I-J	0 / 20	-84.3	-84.3 0.15 (1)	L-I	0 / 557	0.13 (1)	
R-B	-323 / 0	0.0	0.0 0.02 (1)	R-C	-3200 / 0	0.98 (1)	
K-J	-116 / 0	0.0	0.0 0.04 (1)	I-K	-2064 / 0	0.82 (1)	
R-Q	0 / 2692	-28.0	-28.0 0.89 (2)				
Q-P	0 / 2421	-28.0	-28.0 0.86 (2)				
P-O	0 / 2421	-28.0	-28.0 0.86 (2)				
O-N	0 / 1894	-28.0	-28.0 0.53 (2)				
N-M	0 / 1177	-28.0	-28.0 0.46 (2)				
M-L	0 / 1177	-28.0	-28.0 0.46 (2)				
L-K	0 / 928	-28.0	-28.0 0.42 (2)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	25.6	PSF
DL	=	3.0	PSF
BOT CH. LL	=	10.5	PSF
DL	=	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCSC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (1.21")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.28")
ALLOWABLE DEFL.(TL) = $L/360$ (1.21")
CALCULATED VERT. DEFL.(TL) = $L/850$ (0.46")

CSI: TC=0.50 (C-E:1), BC=0.89 (Q-R:2), WB=0.98 (C-R:1), SS=0.23 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1667 822 2284 1656

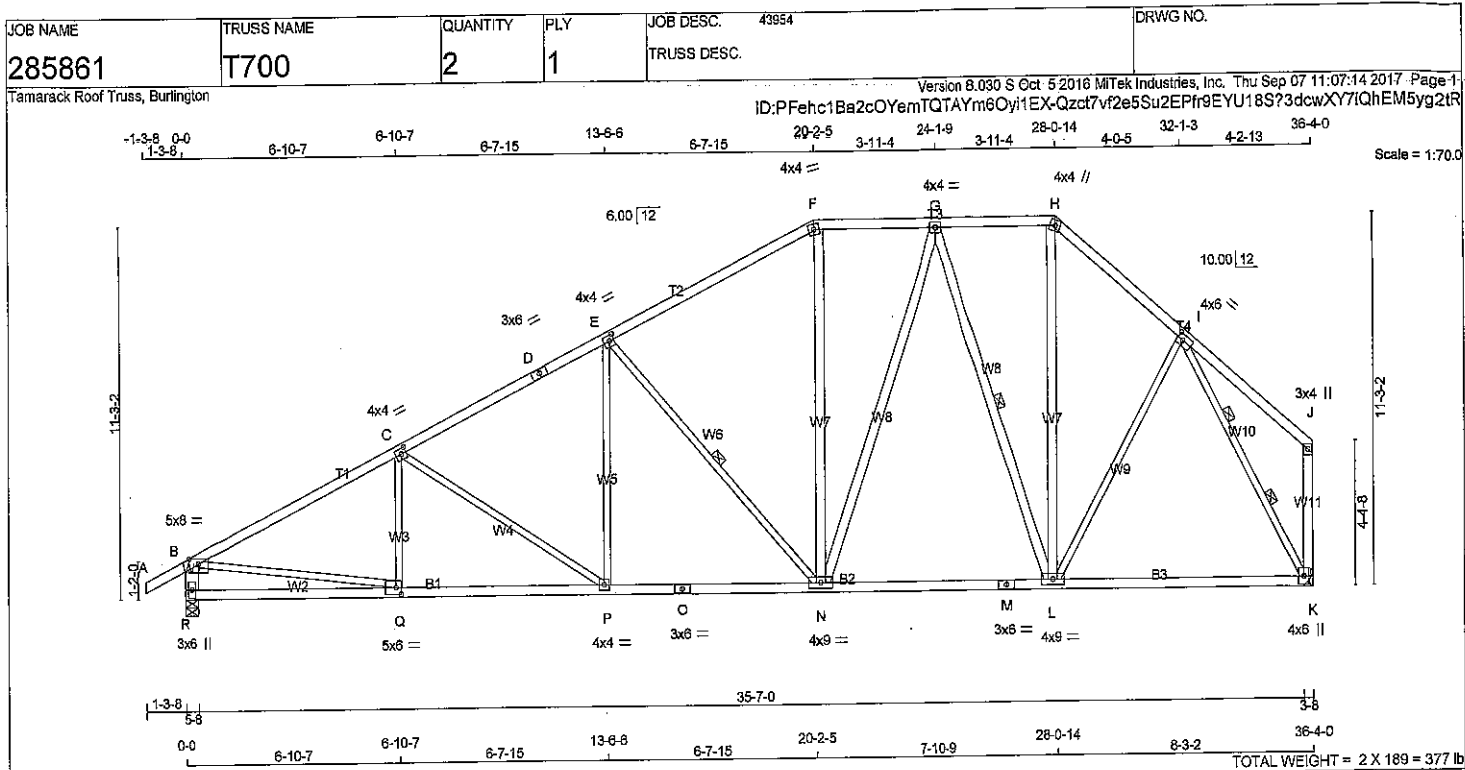
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.84 (P) (INPUT = 1.00)



DRWG NO. TAM 45130-17
STRUCTURAL
COMPONENT ONLY



LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER		DESCR
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF
N - G	2x4	DRY	No.2	SPF
G - L	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMVW-l	MT20	4.0	4.0	2.00	1.75
D	TS-l	MT20	3.0	6.0		
E	TMVW-l	MT20	4.0	4.0	2.00	1.75
F	TTW-m	MT20	4.0	4.0		
G	TMVW-l	MT20	4.0	4.0		
H	TTW-m	MT20	4.0	4.0		
I	TMVW-l	MT20	4.0	6.0	2.00	2.25
J	TMV-p	MT20	3.0	4.0		
K	BMVW-l+p	MT20	4.0	6.0		
L	BMVW-l	MT20	4.0	9.0		
M	BS-l	MT20	3.0	6.0		
N	BMVW-l	MT20	4.0	9.0		
O	BS-l	MT20	3.0	6.0		
P	BMVW-l	MT20	4.0	4.0		
Q	BMVW-l	MT20	5.0	6.0	2.50	2.00
R	BMV-l+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
R	2154	0	2154	0	5-8	5-8
K	2039	0	2039	0	HANGER BY OTHERS	MIN. SEAT SIZE: 3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM.	LIVE			
R	1752	999 / 0	382 / 0	0 / 0	0 / 0	371 / 0	0 / 0
K	1674	930 / 0	382 / 0	0 / 0	0 / 0	363 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.37 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N, G-L

2 LATERAL BRACE(S) AT 1/3 LENGTH OF I-K

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 MAX (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LC1 MAX (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 26	-84.3 -84.3	0.11 (1)	10.00	Q-C	-137 / 199	0.04 (3)
B-C	-3059 / 0	-84.3 -84.3	0.75 (1)	3.37	P-E	-506 / 0	0.66 (1)
C-D	-2607 / 0	-84.3 -84.3	0.57 (1)	3.80	P-E	0 / 523	0.12 (2)
D-E	-2607 / 0	-84.3 -84.3	0.57 (1)	3.80	E-N	-998 / 0	0.78 (1)
E-F	-1890 / 0	-84.3 -84.3	0.53 (1)	4.33	N-F	0 / 478	0.08 (1)
F-G	-1874 / 0	-84.3 -84.3	0.19 (1)	4.96	N-G	0 / 485	0.08 (1)
G-H	-1220 / 0	-84.3 -84.3	0.17 (1)	5.82	G-L	-904 / 0	0.69 (1)
H-I	-1804 / 0	-84.3 -84.3	0.21 (1)	5.04	L-H	0 / 746	0.12 (1)
I-J	0 / 25	-84.3 -84.3	0.22 (1)	10.00	L-I	0 / 422	0.09 (1)
J-K	-2073 / 0	0.0 0.0	0.13 (1)	7.04	B-Q	0 / 2778	0.63 (1)
K-J	-137 / 0	0.0 0.0	0.04 (1)	7.81	I-K	-2057 / 0	0.93 (1)
R-Q	0 / 0	-28.0 -28.0	0.30 (3)	10.00			
Q-P	0 / 2757	-28.0 -28.0	0.62 (2)	10.00			
P-O	0 / 2332	-28.0 -28.0	0.53 (2)	10.00			
O-N	0 / 2332	-28.0 -28.0	0.53 (2)	10.00			
N-M	0 / 1515	-28.0 -28.0	0.65 (2)	10.00			
M-L	0 / 1515	-28.0 -28.0	0.65 (2)	10.00			
L-K	0 / 1002	-28.0 -28.0	0.58 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 3.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 46.1	PSF

SPACING = 24.0 IN./G/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.21")
CALCULATED VERT. DEFL.(LL) = L/999 (0.20")
ALLOWABLE DEFL.(TL) = L/360 (1.21")
CALCULATED VERT. DEFL.(TL) = L/999 (0.33")

CSI: TC=0.75 (B-C:1), BC=0.65 (L-N:2), WB=0.93 (I-K:1), SS=0.24 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR (PSI)	SECTION (PLI)	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1667	822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

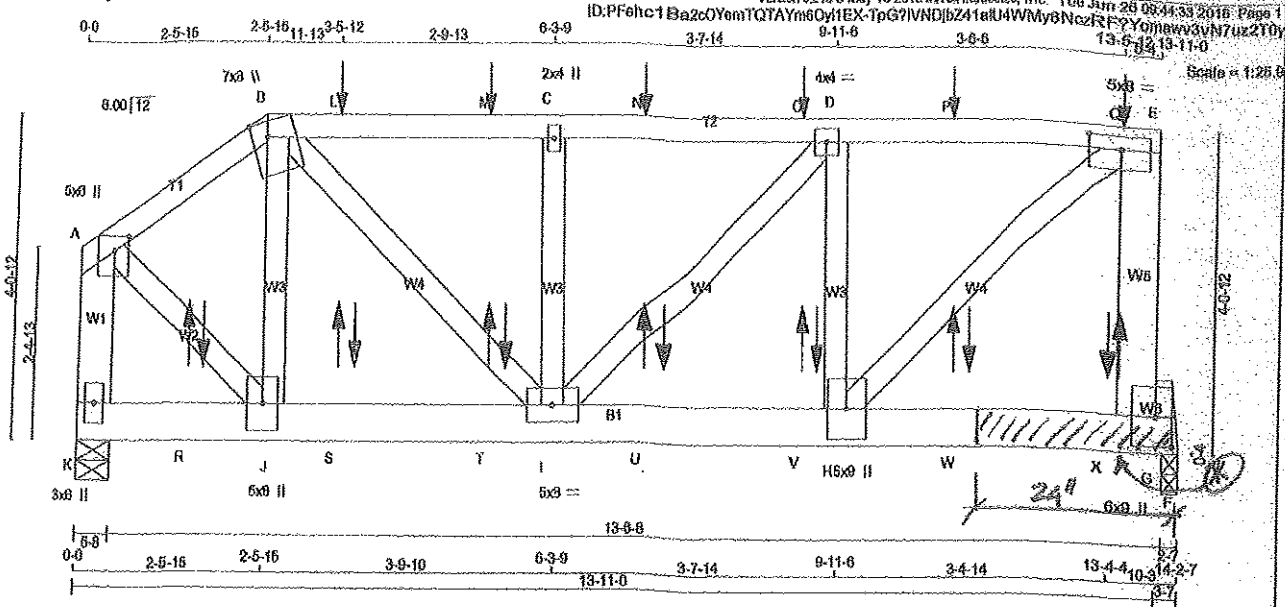
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.69 (O) (INPUT = 1.00)



DRWG NO. TAM 45131-17
STRUCTURAL
COMPONENT ONLY

JOB NAME 285492 Tamarack Roof Truss, Burlington	TRUSS NAME T720	QUANTITY 1	PLY 3	JOB DESC. TRUSS DESC.	DRWG NO.
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TOTAL WEIGHT = 3 X 63 = 248 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No. 2	SPF	
B - E	2x4	DRY	No. 2	SPF	
E - G	2x4	DRY	No. 2	SPF	
K - A	2x4	DRY	No. 2	SPF	
K - F	2x4	DRY	2100F 1.0E	SPF	
G - F	2x4	DRY	No. 2	SPF	

ALL WEBS 2x4 DRY No. 2 SPF

EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122'x3') SPIRAL NAILS		
A - B	12	TOP
B - E	12	SIDE (81.5)
E - G	12	TOP
K - A	12	TOP
BOTTOM CHORDS : (0.122'x3') SPIRAL NAILS		
K - F	7	SIDE (593.4)
WEBS : (0.122'x3') SPIRAL NAILS		
2x4	1	0
2x4	2	0

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

ORDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-D INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table in inches)	W	LEN	Y	X
JT TYPE				
A TMWV+p	MT20	5.0	6.0	2.25 2.25
B TMWV+m	MT20	7.0	6.0	Edge 2.50
C TMWV+w	MT20	2.0	4.0	
D TMWV-t	MT20	4.0	4.0	
E TMWV-i	MT20	5.0	6.0	2.00 4.00
G BMWV+p	MT20	6.0	9.0	4.50 2.50
H BMWV+i	MT20	6.0	9.0	
I BMWV-w	MT20	5.0	6.0	
J BMWV-t	MT20	6.0	6.0	
K BMWV-p	MT20	3.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DWG NO. TAM 71800657
STRUCTURAL
COMPONENT ONLY 1/2

HANGERS NOTES

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT VERT	DOWN	UP	IN-SX
K 7721	0	7721	0
F 8283	0	8283	0

UNFACTORED REACTIONS	1ST LOADE	MAX. MIN. COMPONENT REACTIONS
JT COMBINED	SNOW	LIVE
K 0260	3500/0	1340/0
F 8716	3804/0	1479/0

BEARING MATERIAL TO BE SPF NO. 2 OR BETTER AT JOINT(S) K, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.16 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (7)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LBS)
FR-TO					FR-TO		
A-B	-6013/0	-84.3	-84.3	0.07 (1)	J-B	-14/238	0.01 (1)
B-L	-7833/0	-84.3	-84.3	0.14 (1)	A-J	0/6285	0.34 (1)
L-M	-7833/0	-84.3	-84.3	0.14 (1)	H-E	0/6725	0.52 (1)
M-C	-7833/0	-84.3	-84.3	0.14 (1)	B-I	0/4081	0.22 (1)
C-H	-7833/0	-84.3	-84.3	0.12 (1)	H-D	-1157/0	0.07 (1)
H-O	-7833/0	-84.3	-84.3	0.12 (1)	I-C	-287/0	0.02 (1)
O-D	-7833/0	-84.3	-84.3	0.12 (1)	I-D	0/1188	0.02 (1)
D-P	-7121/0	-84.3	-84.3	0.13 (1)			
P-Q	-7121/0	-84.3	-84.3	0.13 (1)			
Q-E	-7121/0	-84.3	-84.3	0.13 (1)			
E-G	-6776/0	0.0	0.0	0.34 (1)			
K-A	-7233/0	0.0	0.0	0.19 (1)			
K-R	0/0	-28.0	-28.0	0.15 (1)			
R-J	0/0	-28.0	-28.0	0.15 (1)			
J-S	0/4090	-28.0	-28.0	0.23 (1)			
S-T	0/4090	-28.0	-28.0	0.23 (1)			
T-U	0/4090	-28.0	-28.0	0.23 (1)			
U-V	0/7121	-28.0	-28.0	0.30 (1)			
V-H	0/7121	-28.0	-28.0	0.30 (1)			
H-W	0/0	-28.0	-28.0	0.30 (1)			
W-X	0/0	-28.0	-28.0	0.30 (1)			
X-G	0/0	-28.0	-28.0	0.30 (1)			
G-F	0/0	-28.0	-28.0	0.22 (1)			

SPECIFIED CONCENTRATED LOADS (LBS)	LOC.	LCI	MAX.	MAX.	FACE	DIR.	TYPE
L	3-5-12	-1	-1	79	FRONT	VERT	TOTAL
M	6-5-12	-1	-1	74	FRONT	VERT	TOTAL
N	7-5-12	-1	-1	74	FRONT	VERT	TOTAL
O	8-5-12	-1	-1	74	FRONT	VERT	TOTAL
P	11-5-12	-1	-1	74	FRONT	VERT	TOTAL
Q	13-5-12	-1	-1	59	FRONT	VERT	TOTAL
R	1-5-12	4	1	4	FRONT	VERT	TOTAL
R	1-5-12	-1717	-1717	-	BACK	VERT	TOTAL
S	3-5-12	4	1	4	FRONT	VERT	TOTAL
S	3-5-12	-1717	-1717	-	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.6 PSF
DL = 7.0 PSF
TOTAL LOAD = 48.1 PSF

SPACING = 24" O.C.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEF. (LL) = L/360 (0.47')
CALCULATED VERT. DEF. (LL) = L/360 (0.07')
ALLOWABLE DEF. (TL) = L/360 (0.47')
CALCULATED VERT. DEF. (TL) = L/360 (0.11')

CSL TO = 0.34/1.00 (E-G:1), BC = 0.38/1.00 (G-H:1), WD = 0.52/1.00 (E-H:1), SS = 0.03/1.00 (F-G:1)

DOL LUMBER = 1.00 NAIL = 1.00 L.S. BEND = 1.10 COMP = 1.10 SHEAR = 1.10 TENS = 1.10

COMPANION LIVE LOAD FACTOR = 0.50

OTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE (PSI)	SHEAR (PL)	SECTION (PL)
MT20	610	354	1887
MT20	610	354	1887

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

J81 GRIP = 0.08 (E) (INPUT = 0.80)
J81 METAL = 0.52 (E) (INPUT = 1.00)

PROVIDE 24" LONG SCAB 2X6 SPF #2 ALONG MEMBERS (V-A), ON ONE SIDE, USING 2 ROWS OF 3 1/2" SPIRAL NAILS @ 4" C/C. (2 NAILS MIN)

JOB NAME 285492 Tamarack Roof Truss, Burlington	TRUSS NAME T720	QUANTITY 1	PLY 3	JOB DESC. TRUSS DESC.	DRWG NO.
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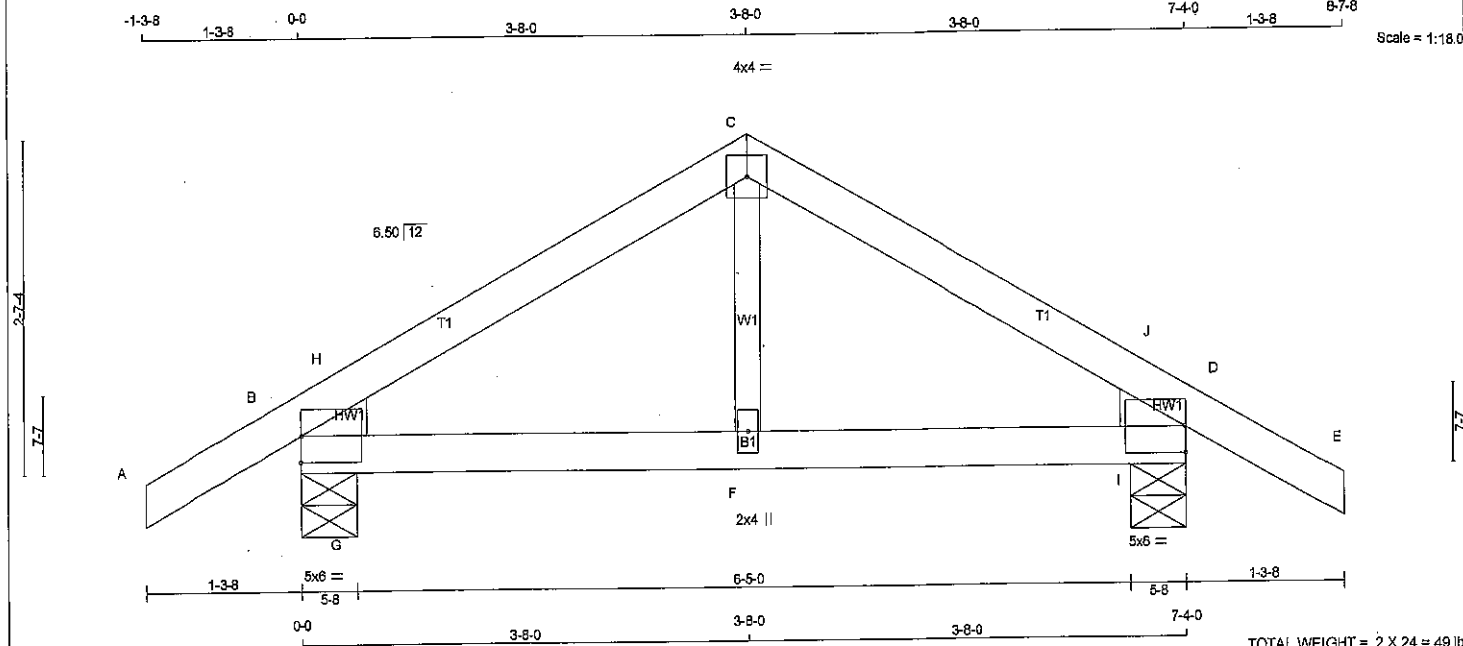
Version 8.210 S May 16 2019 Mtek Industries, Inc. Tue Jun 26 09:14:33 2018 Page 2
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1. THE TRUSS SHALL BE DESIGNED TO SUPPORT THE FOLLOWING LOADS:
 2. DEAD LOAD (DL) - 10 PSF
 3. LIVE LOAD (LL) - 20 PSF
 4. WIND LOAD (WL) - 15 PSF
 5. SNOW LOAD (SL) - 10 PSF
 6. THE TRUSS SHALL BE DESIGNED TO SUPPORT THE FOLLOWING LOADS:
 7. DEAD LOAD (DL) - 10 PSF
 8. LIVE LOAD (LL) - 20 PSF
 9. WIND LOAD (WL) - 15 PSF
 10. SNOW LOAD (SL) - 10 PSF
 11. THE TRUSS SHALL BE DESIGNED TO SUPPORT THE FOLLOWING LOADS:
 12. DEAD LOAD (DL) - 10 PSF
 13. LIVE LOAD (LL) - 20 PSF
 14. WIND LOAD (WL) - 15 PSF
 15. SNOW LOAD (SL) - 10 PSF
 16. THE TRUSS SHALL BE DESIGNED TO SUPPORT THE FOLLOWING LOADS:
 17. DEAD LOAD (DL) - 10 PSF
 18. LIVE LOAD (LL) - 20 PSF
 19. WIND LOAD (WL) - 15 PSF
 20. SNOW LOAD (SL) - 10 PSF
 21. THE TRUSS SHALL BE DESIGNED TO SUPPORT THE FOLLOWING LOADS:
 22. DEAD LOAD (DL) - 10 PSF
 23. LIVE LOAD (LL) - 20 PSF
 24. WIND LOAD (WL) - 15 PSF
 25. SNOW LOAD (SL) - 10 PSF
 26. THE TRUSS SHALL BE DESIGNED TO SUPPORT THE FOLLOWING LOADS:
 27. DEAD LOAD (DL) - 10 PSF
 28. LIVE LOAD (LL) - 20 PSF
 29. WIND LOAD (WL) - 15 PSF
 30. SNOW LOAD (SL) - 10 PSF
 31. THE TRUSS SHALL BE DESIGNED TO SUPPORT THE FOLLOWING LOADS:
 32. DEAD LOAD (DL) - 10 PSF
 33. LIVE LOAD (LL) - 20 PSF
 34. WIND LOAD (WL) - 15 PSF
 35. SNOW LOAD (SL) - 10 PSF
 36. THE TRUSS SHALL BE DESIGNED TO SUPPORT THE FOLLOWING LOADS:
 37. DEAD LOAD (DL) - 10 PSF
 38. LIVE LOAD (LL) - 20 PSF
 39. WIND LOAD (WL) - 15 PSF
 40. SNOW LOAD (SL) - 10 PSF
 41. THE TRUSS SHALL BE DESIGNED TO SUPPORT THE FOLLOWING LOADS:
 42. DEAD LOAD (DL) - 10 PSF
 43. LIVE LOAD (LL) - 20 PSF
 44. WIND LOAD (WL) - 15 PSF
 45. SNOW LOAD (SL) - 10 PSF
 46. THE TRUSS SHALL BE DESIGNED TO SUPPORT THE FOLLOWING LOADS:
 47. DEAD LOAD (DL) - 10 PSF
 48. LIVE LOAD (LL) - 20 PSF
 49. WIND LOAD (WL) - 15 PSF
 50. SNOW LOAD (SL) - 10 PSF
 51. THE TRUSS SHALL BE DESIGNED TO SUPPORT THE FOLLOWING LOADS:
 52. DEAD LOAD (DL) - 10 PSF
 53. LIVE LOAD (LL) - 20 PSF
 54. WIND LOAD (WL) - 15 PSF
 55. SNOW LOAD (SL) - 10 PSF
 56. THE TRUSS SHALL BE DESIGNED TO SUPPORT THE FOLLOWING LOADS:
 57. DEAD LOAD (DL) - 10 PSF
 58. LIVE LOAD (LL) - 20 PSF
 59. WIND LOAD (WL) - 15 PSF
 60. SNOW LOAD (SL) - 10 PSF
 61. THE TRUSS SHALL BE DESIGNED TO SUPPORT THE FOLLOWING LOADS:
 62. DEAD LOAD (DL) - 10 PSF
 63. LIVE LOAD (LL) - 20 PSF
 64. WIND LOAD (WL) - 15 PSF
 65. SNOW LOAD (SL) - 10 PSF
 66. THE TRUSS SHALL BE DESIGNED TO SUPPORT THE FOLLOWING LOADS:
 67. DEAD LOAD (DL) - 10 PSF
 68. LIVE LOAD (LL) - 20 PSF
 69. WIND LOAD (WL) - 15 PSF
 70. SNOW LOAD (SL) - 10 PSF
 71. THE TRUSS SHALL BE DESIGNED TO SUPPORT THE FOLLOWING LOADS:
 72. DEAD LOAD (DL) - 10 PSF
 73. LIVE LOAD (LL) - 20 PSF
 74. WIND LOAD (WL) - 15 PSF
 75. SNOW LOAD (SL) - 10 PSF
 76. THE TRUSS SHALL BE DESIGNED TO SUPPORT THE FOLLOWING LOADS:
 77. DEAD LOAD (DL) - 10 PSF
 78. LIVE LOAD (LL) - 20 PSF
 79. WIND LOAD (WL) - 15 PSF
 80. SNOW LOAD (SL) - 10 PSF
 81. THE TRUSS SHALL BE DESIGNED TO SUPPORT THE FOLLOWING LOADS:
 82. DEAD LOAD (DL) - 10 PSF
 83. LIVE LOAD (LL) - 20 PSF
 84. WIND LOAD (WL) - 15 PSF
 85. SNOW LOAD (SL) - 10 PSF
 86. THE TRUSS SHALL BE DESIGNED TO SUPPORT THE FOLLOWING LOADS:
 87. DEAD LOAD (DL) - 10 PSF
 88. LIVE LOAD (LL) - 20 PSF
 89. WIND LOAD (WL) - 15 PSF
 90. SNOW LOAD (SL) - 10 PSF
 91. THE TRUSS SHALL BE DESIGNED TO SUPPORT THE FOLLOWING LOADS:
 92. DEAD LOAD (DL) - 10 PSF
 93. LIVE LOAD (LL) - 20 PSF
 94. WIND LOAD (WL) - 15 PSF
 95. SNOW LOAD (SL) - 10 PSF
 96. THE TRUSS SHALL BE DESIGNED TO SUPPORT THE FOLLOWING LOADS:
 97. DEAD LOAD (DL) - 10 PSF
 98. LIVE LOAD (LL) - 20 PSF
 99. WIND LOAD (WL) - 15 PSF
 100. SNOW LOAD (SL) - 10 PSF

SPECIFIED CONCENTRATED LOADS (LBS)						
JT	LOC.	LC1	MAX	MAX	FACE	DIR.
T	5-5-12	4	1	4	FRONT	VERT
Y	5-8-4	-1717	-1717	—	BACK	VERT
U	7-5-12	4	1	4	FRONT	VERT
U	7-8-4	-1639	-1639	—	BACK	VERT
V	9-5-12	4	1	4	FRONT	VERT
V	9-8-4	-1639	-1639	—	BACK	VERT
W	11-5-12	4	1	4	FRONT	VERT
W	11-8-4	-1639	-1639	—	BACK	VERT
X	13-4-4	-1642	-1642	—	BACK	VERT
X	13-5-12	4	1	4	FRONT	VERT
						TOTAL



DWG NO. TAM **718 00651**
 STRUCTURAL
 COMPONENT ONLY **7/2**



LUMBER
 N. L. G. A. RULES
 CHORDS SIZE DRY LUMBER DESCR. SPF
 A - C 2x4 DRY No.2 SPF
 C - E 2x4 DRY No.2 SPF
 B - D 2x4 DRY No.2 SPF
 ALL WEBS 2x3 DRY No.2 SPF
 DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-I	MT20	5.0	6.0		
C	TTW-p	MT20	4.0	4.0		
D	TMBH1-I	MT20	5.0	6.0		
F	BMV+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX	HEEL WEDGE
	VERT	HORZ	DOWN	HORZ			
B	526	0	526	0	5-8	5-8	2x4 L
D	526	0	526	0	5-8	5-8	2x4 R

UNFACTORED REACTIONS								
JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS				WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE				
B	416	257 / 0	77 / 0	0 / 0	0 / 0	82 / 0	0 / 0	
D	416	257 / 0	77 / 0	0 / 0	0 / 0	82 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 17	-84.3 -84.3	0.11 (1)	10.00	F-C	0 / 192	0.04 (2)
B-H	-459 / 0	-84.3 -84.3	0.07 (1)	6.25	G-H	-55 / 132	0.00 (1)
H-C	-410 / 0	-84.3 -84.3	0.11 (1)	6.25	I-J	-55 / 132	0.00 (1)
C-J	-410 / 0	-84.3 -84.3	0.11 (1)	6.25			
J-D	-459 / 0	-84.3 -84.3	0.07 (1)	6.25			
D-E	0 / 17	-84.3 -84.3	0.11 (1)	10.00			
B-G	0 / 352	-28.0 -28.0	0.10 (1)	10.00			
G-F	0 / 352	-28.0 -28.0	0.13 (1)	10.00			
F-I	0 / 352	-28.0 -28.0	0.13 (1)	10.00			
I-D	0 / 352	-28.0 -28.0	0.10 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 25.6 PSF
 DL = 3.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 46.1 PSF

SPACING = 240 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF CBC 2012, CBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.24")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
 ALLOWABLE DEFL.(TL) = L/360 (0.24")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.11 (A-B:1), BC=0.13 (F-G:1), WB=0.04 (C-F:2), SSI=0.11 (C-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1657 822 2284 1658

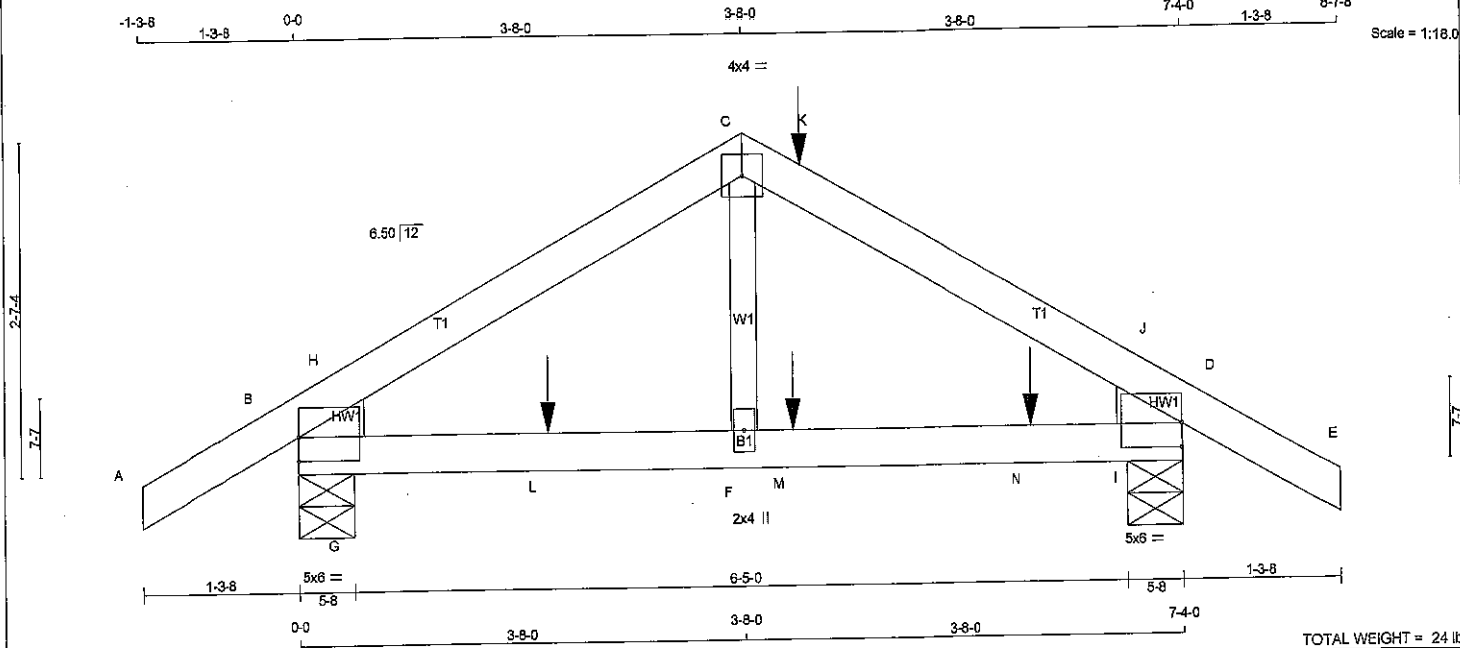
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (D) (INPUT = 0.90)
 JSI METAL= 0.12 (D) (INPUT = 1.00)



DWG NO. TAM 4513317
 STRUCTURAL
 COMPONENT ONLY



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2
SPF
DRY: SEASONED LUMBER.

PLATES (tablets in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-I	MT20	5.0	6.0	2.25	
C	TTW-p	MT20	4.0	4.0		
D	TMBH1-I	MT20	5.0	6.0	2.25	
F	BMW+w	MT20	2.0	4.0		

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 26.9 lbs FACTORED DOWN AT 4-0-12 ON TOP CHORD, AND 8.5 lbs FACTORED DOWN AT 2-0-12, AND 6.5 lbs FACTORED DOWN AT 4-0-12, AND 6.5 lbs FACTORED DOWN AT 6-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG	HEEL WEDGE
B	543	0	5-8	5-8	2x4 L
D	547	0	5-8	5-8	2x4 R

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	430	265 / 0	81 / 0	0 / 0	0 / 0	0 / 0	85 / 0	0 / 0
D	434	268 / 0	82 / 0	0 / 0	0 / 0	0 / 0	86 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)
FR-TO		FROM	TO	LENGTH	FR-TO		
A-B	0 / 17	-84.3	-84.3	0.12 (1)	10.00	F-C	0 / 203
B-H	-496 / 0	-84.3	-84.3	0.10 (1)	6.25	G-H	-43 / 141
H-C	-438 / 0	-84.3	-84.3	0.13 (1)	6.25	I-J	-50 / 142
C-K	-433 / 0	-84.3	-84.3	0.14 (1)	6.25		
K-J	-433 / 0	-84.3	-84.3	0.14 (1)	6.25		
J-D	-493 / 0	-84.3	-84.3	0.10 (1)	6.25		
D-E	0 / 17	-84.3	-84.3	0.12 (1)	10.00		
B-G	0 / 377	-28.0	-28.0	0.11 (1)	10.00		
G-L	0 / 377	-28.0	-28.0	0.15 (1)	10.00		
L-F	0 / 377	-28.0	-28.0	0.15 (1)	10.00		
F-M	0 / 377	-28.0	-28.0	0.15 (1)	10.00		
M-N	0 / 377	-28.0	-28.0	0.15 (1)	10.00		
N-I	0 / 377	-28.0	-28.0	0.15 (1)	10.00		
I-D	0 / 377	-28.0	-28.0	0.11 (1)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
K	4-0-12	-27	-27	-	BACK	VERT	TOTAL
L	2-0-12	-4	-4	-	BACK	VERT	TOTAL
M	4-0-12	-4	-4	-	BACK	VERT	TOTAL
N	6-0-12	-4	-4	-	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TFC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360 (0.24")$
CALCULATED VERT. DEFL.(LL) = $L/999 (0.01")$
ALLOWABLE DEFL.(TL) = $L/360 (0.24")$
CALCULATED VERT. DEFL.(TL) = $L/999 (0.01")$

CSI: TC=0.14 (C-J:1), BC=0.15 (F-G:1), WB=0.05 (C-F:2), SSI=0.12 (C-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1657 822 2284 1858

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (D) (INPUT = 0.90)
JSI METAL= 0.12 (D) (INPUT = 1.00)



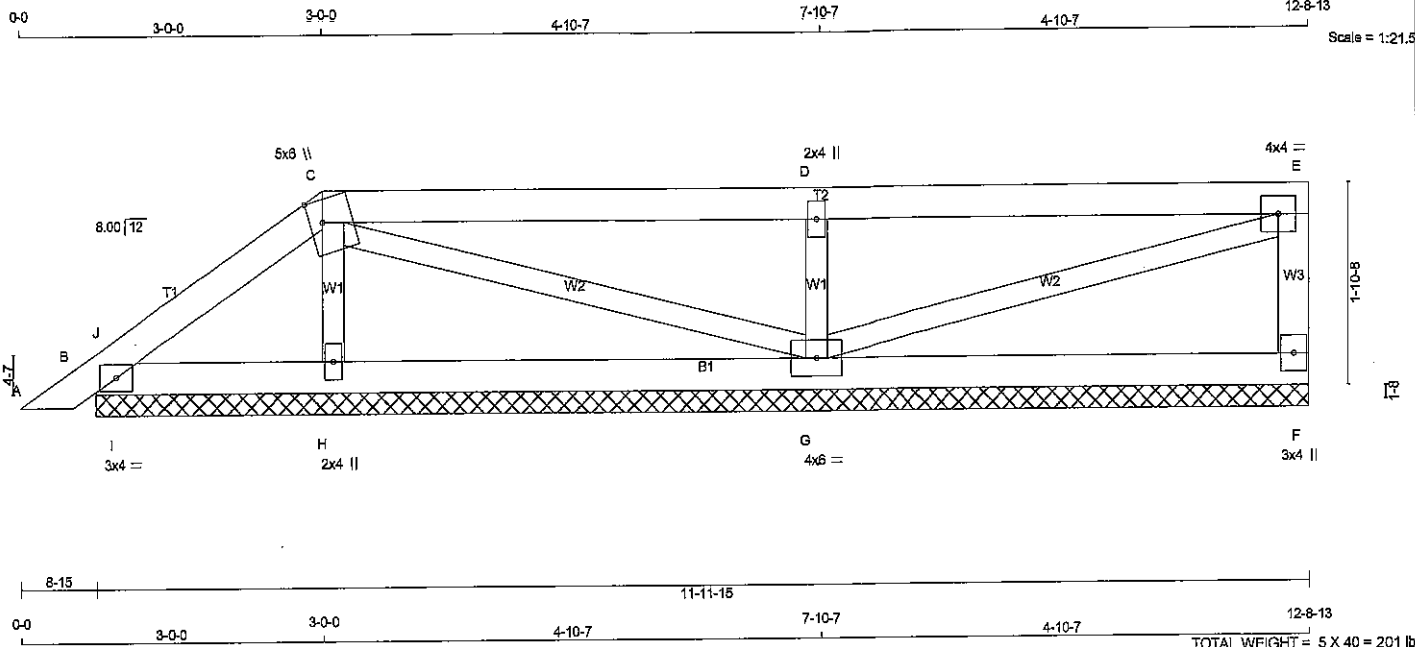
DWG NO. TAM 45734-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
285861	P6	5	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 5 X 40 = 201 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - m	2x4	DRY	No.2	SPF
F - m	2x4	DRY	No.2	SPF
B - T	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMB1-I	MT20	3.0	4.0		
C TTWW+m	MT20	5.0	6.0	2.50	1.50
D TMW+w	MT20	2.0	4.0		
E TMWV-t	MT20	4.0	4.0		
F BMV1+p	MT20	3.0	4.0		
G BMWVW1-t	MT20	4.0	6.0		
H BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
F	210	0	210	0	11-11-15	11-11-15
B	194	0	194	0	11-11-15	11-11-15
H	321	0	321	0	11-11-15	11-11-15
G	677	0	677	0	11-11-15	11-11-15

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	173	95/0	40/0	0/0	0/0	38/0	0/0
B	142	106/0	14/0	0/0	0/0	22/0	0/0
H	278	131/0	79/0	0/0	0/0	88/0	0/0
G	550	315/0	119/0	0/0	0/0	116/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MAX. FACTORED		FACTORED			MAX. FACTORED				
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX. CSI (LG)	MAX. UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX. CSI (LG)		
FR-TO		FROM TO			FR-TO				
A-B	0 / 14	-84.3	-84.3	0.03 (1)	10.00	H-C	-198 / 0	0.03 (1)	
B-J	-32 / 0	-84.3	-84.3	0.01 (1)	6.25	C-G	-27 / 0	0.01 (1)	
J-C	-49 / 0	-84.3	-84.3	0.05 (1)	6.25	G-D	-512 / 0	0.07 (1)	
C-D	0 / 0	-84.3	-84.3	0.34 (1)	10.00	G-E	0 / 0	0.00 (1)	
D-E	0 / 0	-84.3	-84.3	0.34 (1)	10.00	I-J	-121 / 0	0.00 (1)	
F-E	-156 / 0	0.0	0.0	0.02 (1)	7.81				
B-I	0 / 38	-28.0	-28.0	0.05 (1)	10.00				
I-H	0 / 38	-28.0	-28.0	0.09 (2)	10.00				
H-G	0 / 25	-28.0	-28.0	0.17 (2)	10.00				
G-F	0 / 0	-28.0	-28.0	0.17 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD	=	48.1	PSF	

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 089-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.34 (C-D:1), BC=0.17 (G-H:2), WB=0.07 (D-G:1), SSI=0.20 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

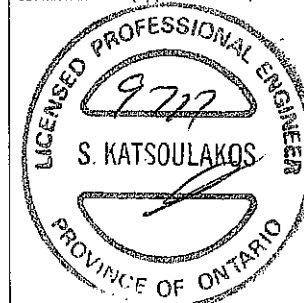
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	618	354	1687 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

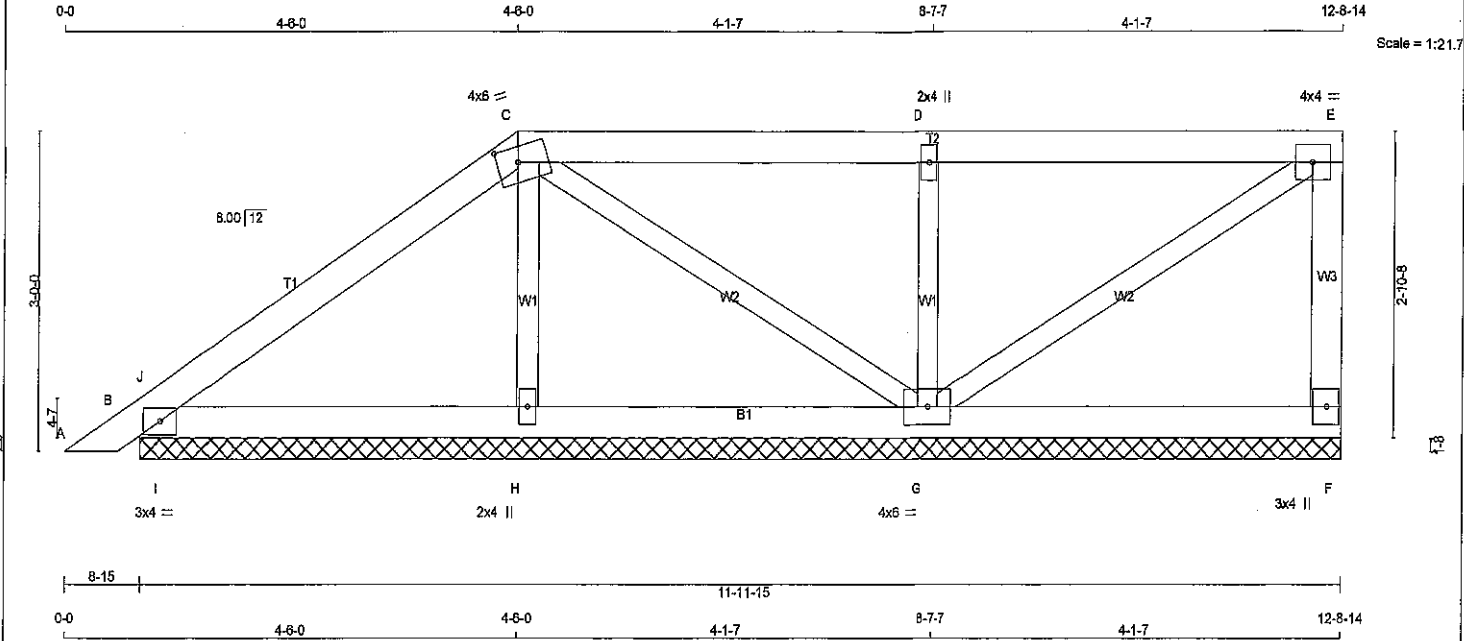
JSI GRIP= 0.27 (D) (INPUT = 0.90)
JSI METAL= 0.09 (D) (INPUT = 1.00)



DWG NO. TAM 45139-17
STRUCTURAL
COMPONENT ONLY

JOB NAME 285861	TRUSS NAME P7	QUANTITY 2	PLY 1	JOB DESC. 43954	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Thu Sep 07 11:06:55 2017 Page 1	

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LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - E	2x4	DRY No.2	SPF
F - E	2x4	DRY No.2	SPF
B - F	2x4	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW-m	MT20	4.0	6.0	1.75	2.50
D	TMW+w	MT20	2.0	4.0		
E	TMW-t	MT20	4.0	4.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWV1-t	MT20	4.0	6.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
F	181	0	181	0	11-11-15	11-11-15
B	301	0	301	0	11-11-15	11-11-15
H	328	0	328	0	11-11-15	11-11-15
G	592	0	592	0	11-11-15	11-11-15

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX / MIN. COMPONENT REACTIONS		PERM. LIVE	WIND	DEAD	SOIL
		SNOW	LIVE				
F	150	81 / 0	36 / 0	0 / 0	0 / 0	33 / 0	0 / 0
B	232	153 / 0	37 / 0	0 / 0	0 / 0	42 / 0	0 / 0
H	285	193 / 0	82 / 0	0 / 0	0 / 0	70 / 0	0 / 0
G	478	281 / 0	97 / 0	0 / 0	0 / 0	98 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, H, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING									
TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. FACTORED CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM	TO			FR-TO			
A-B	0 / 14	-84.3	-84.3	0.03 (1)	10.00	H-C	-178 / 0	0.03 (1)	
B-J	-73 / 0	-84.3	-84.3	0.04 (1)	6.25	C-G	-75 / 0	0.03 (1)	
J-C	-91 / 0	-84.3	-84.3	0.15 (1)	6.25	G-D	-433 / 0	0.07 (1)	
C-D	-4 / 0	-84.3	-84.3	0.24 (1)	10.00	G-E	0 / 5	0.00 (3)	
D-E	-4 / 0	-84.3	-84.3	0.24 (1)	10.00	I-J	-263 / 74	0.00 (1)	
F-E	-133 / 0	0.0	0.0	0.02 (1)	7.81				
B-I	0 / 71	-28.0	-28.0	0.13 (1)	10.00				
I-H	0 / 71	-28.0	-28.0	0.13 (1)	10.00				
H-G	0 / 64	-28.0	-28.0	0.12 (2)	10.00				
G-F	0 / 0	-28.0	-28.0	0.12 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/G

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.24 (C-D:1), BC=0.13 (H-I:1), WB=0.07 (D-G:1), SS=0.22 (B-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (D) (INPUT = 0.90)
JSI METAL= 0.08 (D) (INPUT = 1.00)



DWG NO. TAM 45142-17
STRUCTURAL
COMPONENT ONLY

LUL/LUS/LJS/HUS/HHUS/HGUS

SIMPSON
Strong-Tie

Standard and Double-Shear Joist Hangers



This product is preferable to similar connectors because of
a) easier installation, b) higher capacities, c) lower installed
cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 258–259.

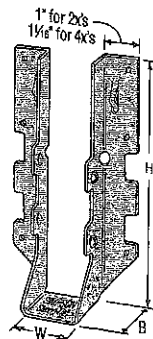
Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 20–24.

Installation:

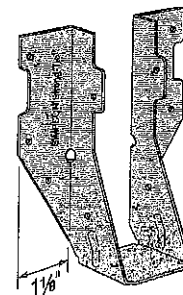
- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

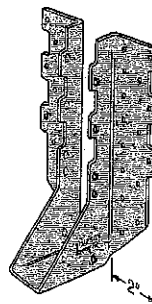
- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on p. 126.



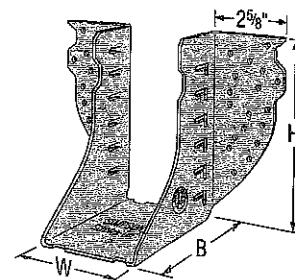
✓ LUS28



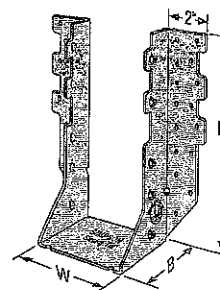
LU26L



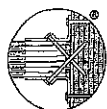
✓ HUS210
(HUS26, HUS28,
and HHUS similar)



✓ HGUS28-2



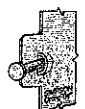
✓ HHUS210-2



Double-Shear
Nailing
Top View

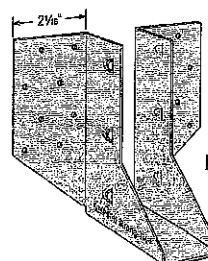
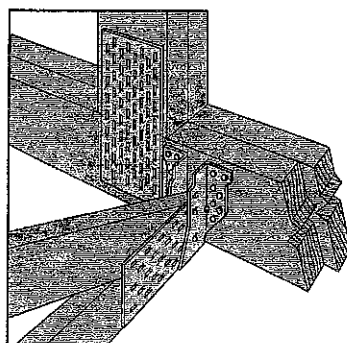


Double-Shear
Nailing
Side View;
Do not
bend tab



Dome Double-Shear
Nailing Side View
(available on
some models)
U.S. Patent 5,803,580

**Typical HUS26
Installation
with Reduced
Heel Height**
(Truss Designer
to provide
fastener quantity
for connecting
multiple members
together)



LJS26DS

Plated Truss Connectors



LUL/LUS/LJS/HUS/HHUS/HGUS

HHUS/HGUS

See Hanger Options information on pp. 125–127.

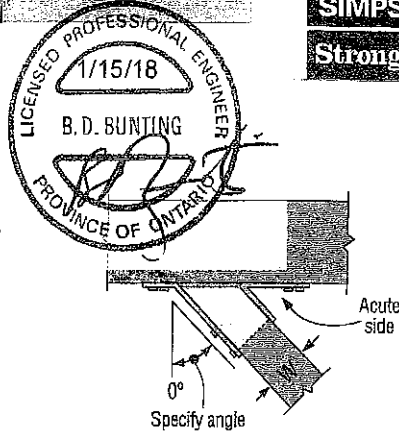
HHUS — Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.62 of the table value
- The joist must be bevel-cut to allow for double-shear nailing

HGUS — Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Joist	Down Resistance	Uplift
W < 2"	Bevel or square cut	0.62 of table value	0.46 of table value
2" < W < 6"	Bevel cut	0.67 of table value	0.41 of table value
2" < W < 6"	Square cut	0.46 of table value	0.41 of table value
W > 6"	Bevel cut	0.75 of table value	0.41 of table value



Top View HHUS Hanger

Skewed Right

(joist must be bevel cut)
All joist nails installed on the outside angle (non-acute side).

Standard and Double-Shear Joist Hangers (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.)				Fasteners			Factored Resistance			
		W	H	B	d _e ³	Header	Joist	D-Fir-L		S-P-F		
								Uplift	Normal	Uplift	Normal	
								(K _D = 1.15)	(K _D = 1.00)	(K _D = 1.15)	(K _D = 1.00)	
								lb.	lb.	lb.	lb.	
Single 2x Sizes												
LUS24	18	1 1/8	3 1/2	1 1/4	2 1/4	(4) 10d	(2) 10d	710	1625	645	1155	
								3.16	7.23	2.87	5.14	
LU24L	22	1 1/8	3	1 1/4	2 1/8	(4) 10d	(2) 10d x 1 1/2"	360	1020	320	725	
								1.60	4.54	1.42	3.22	
LU26L	22	1 1/8	5	1 1/4	4 1/4	(6) 10d	(4) 10d x 1 1/2"	720	1605	645	1140	
								3.20	7.14	2.87	5.07	
LUS26	18	1 1/8	4 1/4	1 1/4	3 1/4	(4) 10d	(4) 10d	1420	2170	1290	1630	
								6.32	9.85	5.74	7.25	
HUS26	16	1 1/8	5 1/4	3	3 1/8	(14) 16d	(6) 16d	2705	4940	2065	3875	
								11.30	21.37	9.20	17.24	
LJS26DS	18	1 1/8	5	3 1/2	4 1/4	(16) 16d	(6) 16d	2055	4285	1460	4115	
								9.34	18.97	6.49	18.31	
HGUS26	12	1 1/8	5 1/4	5	4 1/4	(20) 16d	(8) 16d	2685	6625	2685	5700	
								11.96	29.51	11.96	25.35	
LU28L	20	1 1/8	6 1/4	1 1/4	5 1/4	(8) 10d	(6) 10d x 1 1/2"	1140	2185	1020	1550	
								5.07	9.72	4.54	6.89	
LUS28	18	1 1/8	6 1/4	1 1/4	3 3/4	(6) 10d	(4) 10d	1420	2520	1290	1790	
								6.32	11.21	5.74	7.96	
HUS28	16	1 1/8	7 1/4	3	6 1/8	(22) 16d	(8) 16d	3605	5365	2675	4345	
								16.04	23.86	11.90	19.33	
HGUS28	12	1 1/8	7 1/4	5	6 1/4	(36) 16d	(12) 16d	3310	7675	3310	6900	
								14.74	34.19	14.74	30.73	
LU210L	20	1 1/8	8	1 1/4	7 1/4	(10) 10d	(6) 10d x 1 1/2"	1140	2495	1020	1770	
								5.07	11.10	4.54	7.87	
LUS210	18	1 1/8	7 1/4	1 1/4	3 3/4	(8) 10d	(4) 10d	1420	2785	1290	2210	
								6.32	12.39	5.74	9.83	

1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.
3. d_e is the distance from the bearing seat to the top joist nail.
4. Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-C-N10TRSSCN and/or see installation notes.
5. Nails: 16d = 0.162" dia. x 3 1/2" long. See pp. 27–28 for other nail sizes and information.

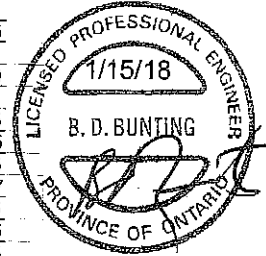
Face-Mount Hangers

SIMPSON
Strong-Tie

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

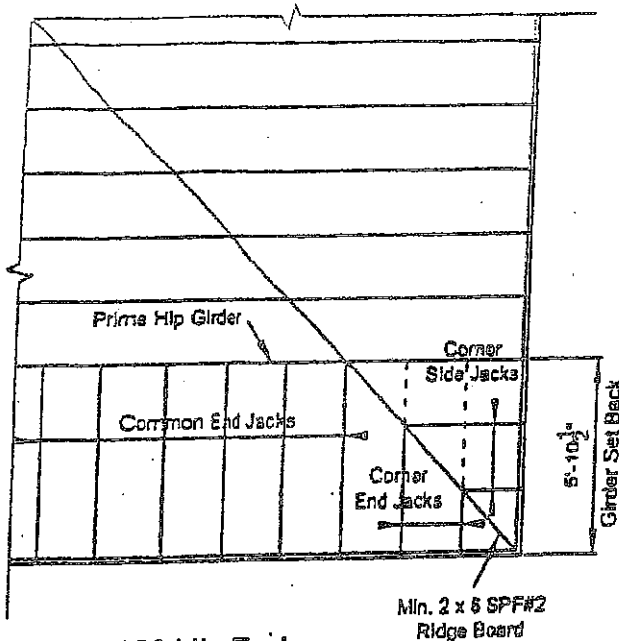
Model No.	Ga.	Dimensions (In.)				Fasteners		Factored Resistance			
		W	H	B	d _c ³	Header	Joist	D-Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
								(K _D = 1.15)	(K _D = 1.00)	(K _D = 1.15)	(K _D = 1.00)
								lb.	lb.	lb.	lb.
								kN	kN	kN	kN
Double 2x Sizes											
LUS24-2	18	3 1/8	3 1/8	2	1 1/4	(4) 16d	(2) 16d	835	2020	590	1435
								3.71	8.99	2.62	6.38
LUS26-2	18	3 1/8	4 1/8	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
								7.65	11.54	6.87	8.54
HHUS26-2	14	3 1/8	5 1/8	3	3 1/8	(14) 16d	(6) 16d	2850	7335	2065	5205
								12.68	32.63	9.20	23.15
HGUS26-2	12	3 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
								7.65	14.79	6.87	11.45
HHUS28-2	14	3 1/8	7 1/8	3	6 1/8	(22) 16d	(8) 16d	3765	8940	2675	6345
								16.75	39.77	11.90	28.22
HGUS28-2	12	3 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
								11.48	20.02	10.32	14.21
HHUS210-2	14	3 1/8	9 1/8	3	8	(30) 16d	(10) 16d	4670	9660	4235	7000
								20.77	42.97	18.84	31.14
HGUS210-2	12	3 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	6840	14015	4855	10270
								30.43	62.34	21.60	45.69
Triple 2x Sizes											
HGUS26-3	12	4 1/8	5 1/2	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
HGUS28-3	12	4 1/8	7 1/4	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
HHUS210-3	14	4 1/8	9	3	7 1/8	(30) 16d	(10) 16d	4670	9670	4235	6865
								20.77	43.02	18.84	30.54
HGUS210-3	12	4 1/8	9 1/4	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400
								30.43	65.14	21.60	46.26
Quadruple 2x Sizes											
HGUS26-4	12	6 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
HGUS28-4	12	6 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
HHUS210-4	14	6 1/8	8 1/8	3	7 1/8	(30) 16d	(10) 16d	4670	10155	4235	7210
								20.77	45.17	18.84	32.07
HGUS210-4	12	6 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400
								30.43	65.14	21.60	46.26
HGUS212-4	12	6 1/8	10 1/8	4	10 1/8	(56) 16d	(20) 16d	7640	14995	5425	10645
								33.98	66.70	24.13	47.35
HGUS214-4	12	6 1/8	12 1/8	4	11 1/8	(66) 16d	(22) 16d	10130	16400	7195	11645
								45.06	72.95	32.00	51.80
4x Sizes											
LUS46	18	3 1/8	4 1/8	2	3 1/8	(4) 16d	(4) 16d	1720	2595	1545	1920
								7.65	11.54	6.87	8.54
HHUS46	14	3 1/8	5 1/8	3	3 1/8	(14) 16d	(6) 16d	2540	7335	2065	5205
								11.30	32.63	9.20	23.15
HGUS46	12	3 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
LUS48	18	3 1/8	6 1/8	2	3 1/8	(6) 16d	(4) 16d	1720	3325	1545	2575
								7.65	14.79	6.87	11.45
HHUS48	14	3 1/8	7 1/8	3	6 1/8	(22) 16d	(8) 16d	3765	8940	2675	6345
								16.75	39.77	11.90	28.22
HGUS48	12	3 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
LUS410	18	3 1/8	8 1/8	2	5 1/8	(8) 16d	(6) 16d	2580	4500	2320	3195
								11.48	20.02	10.32	14.21
HGUS410	12	3 1/8	9	4	8 1/8	(46) 16d	(16) 16d	6840	14015	4855	10270
								30.43	62.34	21.60	45.69
HGUS412	12	3 1/8	10 1/8	4	10 1/8	(56) 16d	(20) 16d	7640	14995	5425	10645
								33.98	66.70	24.13	47.35
HGUS414	12	3 1/8	12 1/8	4	11 1/8	(66) 16d	(22) 16d	10130	16400	7195	11645
								45.06	72.95	32.00	51.80



Plated Truss Connectors

MICRO CITY ENGINEERING SERVICES INC.

TEL: (519) 287-2242



45° Hip End

LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2

BOTTOM CHORD : 2 x 4 SPF#2

WEBS : 2 x 3 SPF#2

UNLESS OTHERWISE SHOWN

DESIGN LOAD:

TOP CHORD LIVE LOAD : 34.8 P.S.F.

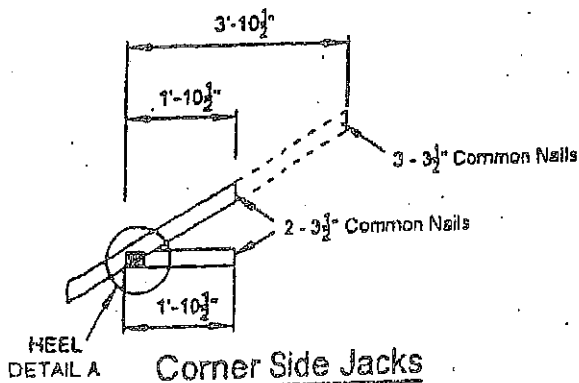
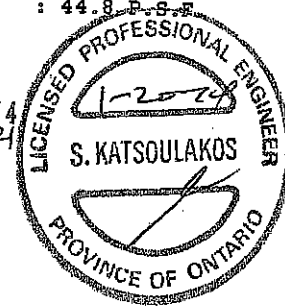
TOP CHORD DEAD LOAD : 3.0 P.S.F.

BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.

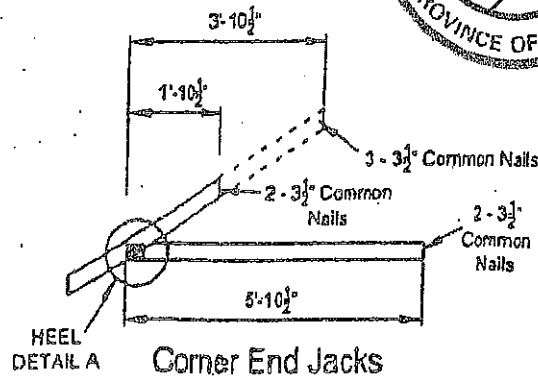
BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD : 44.8 P.S.F.

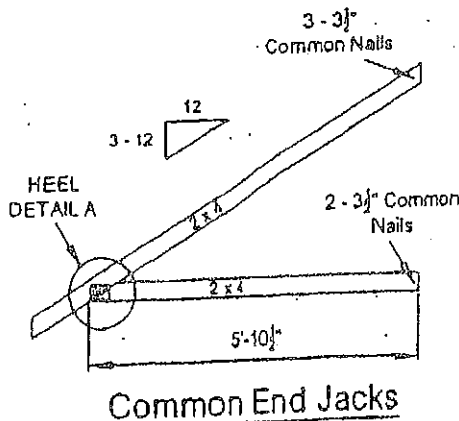
DWG NO TAM 3495.14
STRUCTURAL
COMPONENT ONLY



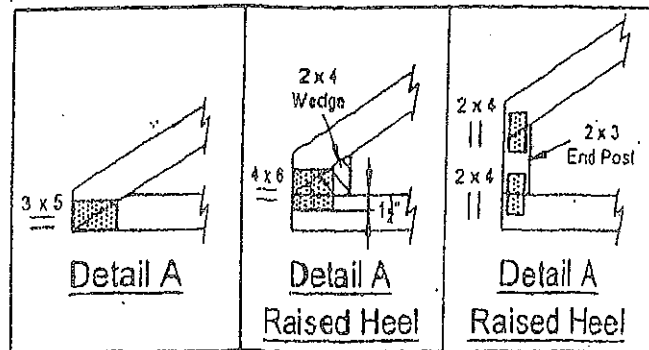
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

MICRO CITY ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

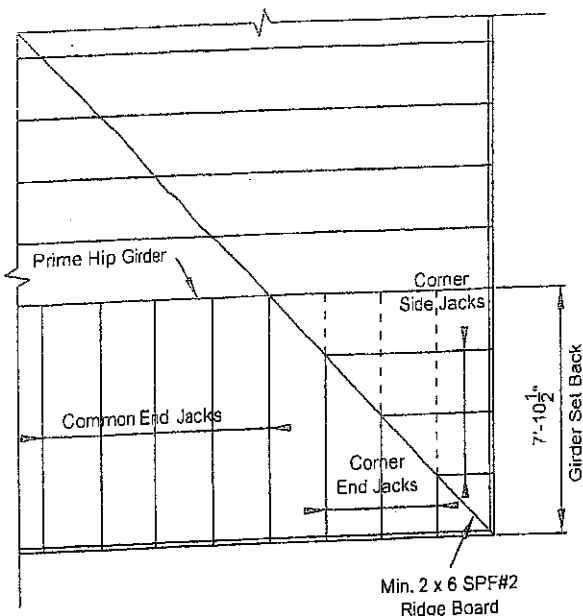
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

DESIGN LOAD:

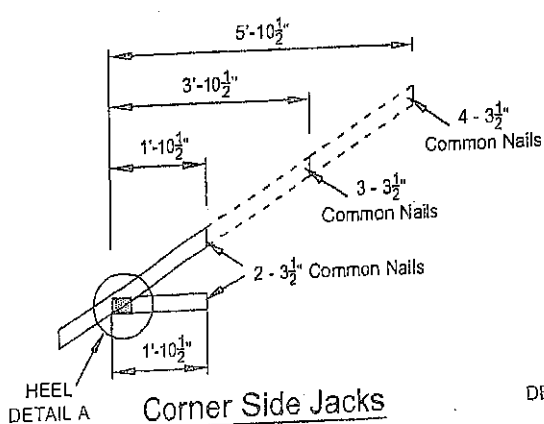
TOP CHORD LIVE LOAD : 40.5 P.S.F.
TOP CHORD DEAD LOAD : 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD : 50.5 P.S.F.

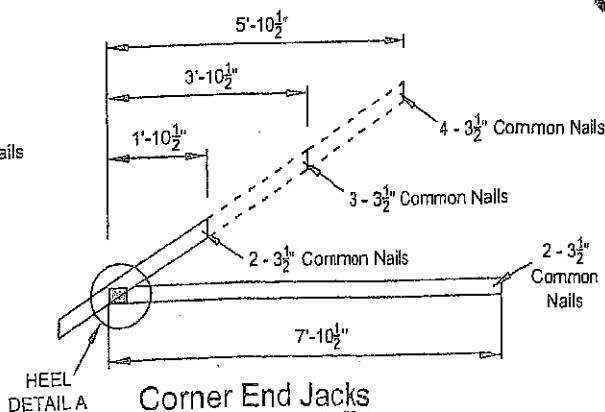


45° Hip End

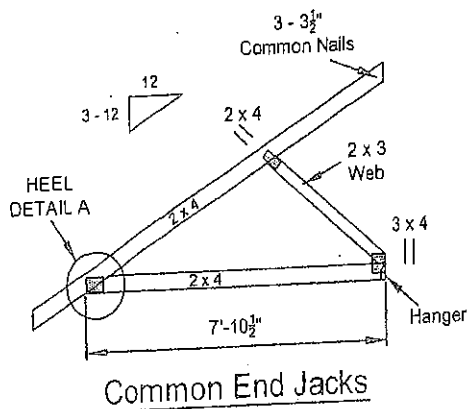
DVO NO TAM 3504.1
STRUCTURAL
COMPONENT ONLY



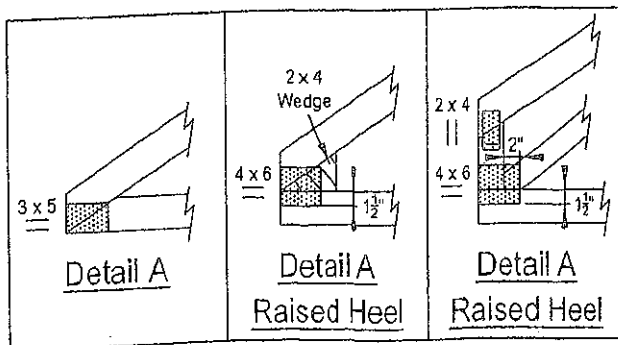
Corner Side Jacks



Corner End Jacks



Common End Jacks



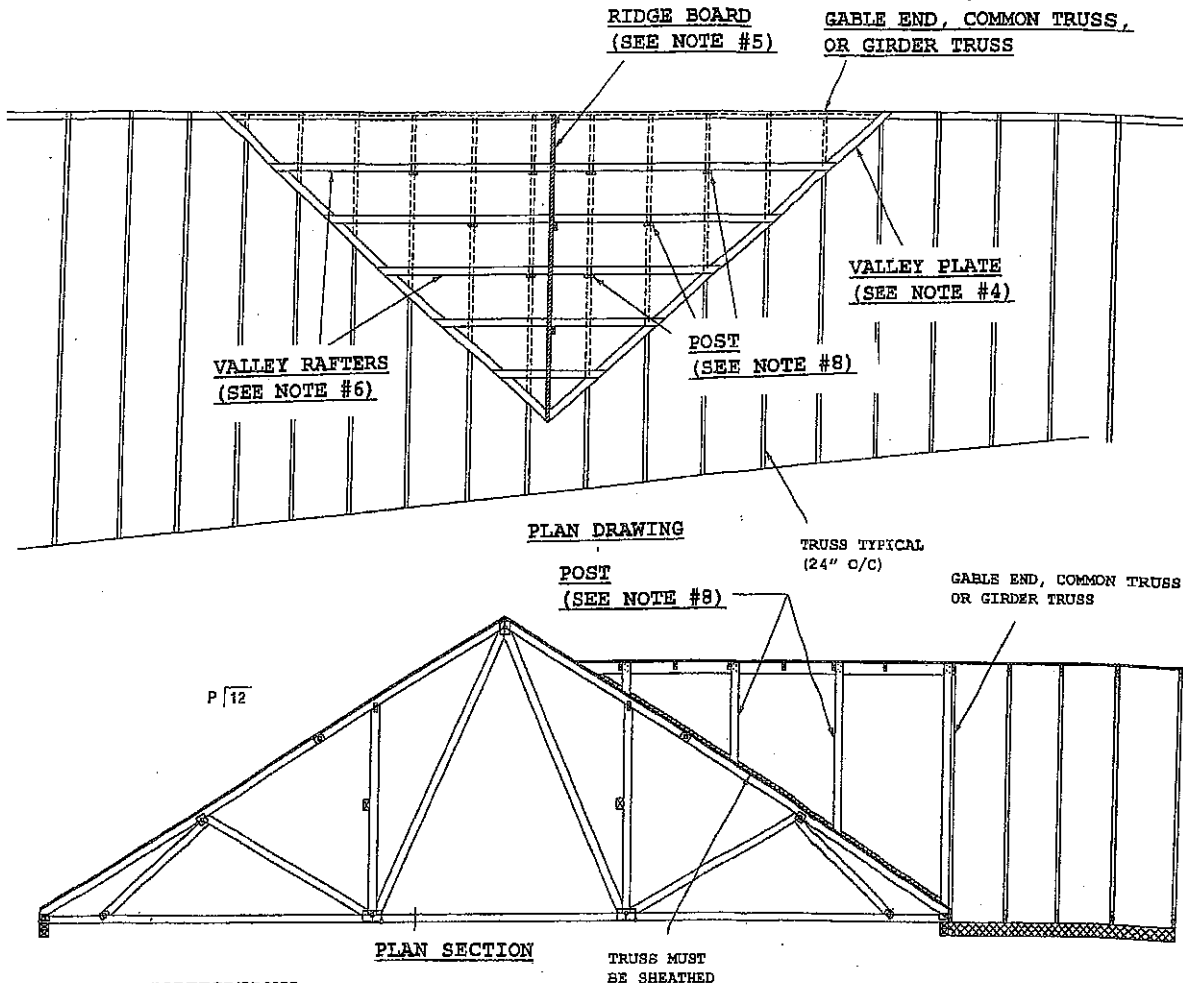
NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

MICRO CITY ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

CONVENTIONAL VALLEY FRAMING DETAIL



GENERAL SPECIFICATIONS:

- (1) WITH THE BASE TRUSSES ERECTED (INSTALLED), APPLY SHEATHING TOP CHORD OF SUPPORTING (BASE) TRUSSES.
- (2) BRACE BOTTOM CHORD AND WEB MEMBERS AS PER PRE-ENGINEERED TRUSS DESIGNS.
- (3) DEFINE VALLEY RIDGE BY RUNNING A LEVEL STRING FROM THE INTERSECTING RIDGE OF THE (a) GABLE END, (b) GIRDER TRUSS OR (c) COMMON TRUSS TO THE ROOF SHEATHING.
- (4) INSTALL 2 X 6 VALLEY PLATES ON FLAT. FASTEN TO EACH SUPPORTING TRUSS WITH (2) 16d (3.5" X 0.131") NAILS.
- (5) SET A 2 X 6 #2 RIDGE BOARD (MAX. 10'-0" RIDGE) OR 2 X 8 #2 SPF RIDGE BOARD (MAX. 20'-0" RIDGE). SUPPORT RIDGE BOARD WITH 2 X 4 POSTS SPACED 48" O/C. BEVEL BOTTOM OF POST TO SET EVENLY ON THE SHEATHING. FASTEN POST TO RIDGE WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST TO ROOF SHEATHING WITH (3) 10d (3" X 0.131") TOE-NAILS.
- (6) FRAME VALLEY RAFTERS FROM VALLEY PLATE TO RIDGE BOARD. MAXIMUM RAFTER SPACING IS 24" O/C. FASTEN VALLEY RAFTER TO RIDGE BEAM WITH (3) 16d (3.5" X 0.131") TOE-NAILS. FASTEN VALLEY RAFTER TO VALLEY PLATE WITH (3) 16d (3.5" X 0.131") TOE-NAILS.
- (7) SUPPORT THE VALLEY RAFTERS WITH 2 X 4 POSTS AT 48" O/C (OR LESS) ALONG EACH RAFTER. INSTALL POSTS IN A STAGGERED PATTERN AS SHOWN ON PLAN DRAWING. ALIGN POSTS WITH TRUSSES BELOW. FASTEN VALLEY RAFTER TO POST WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST THROUGH SHEATHING TO SUPPORTING TRUSSES WITH (2) 16d (3.5" X 0.131") NAILS.
- (8) POSTS SHALL BE 2 X 4 #2 SPF OR BETTER. POSTS EXCEEDING 75" IN HEIGHT SHALL BE INCREASED TO 4 X 4 #2 SPF, OR BETTER, OR BE PRE-ASSEMBLED TWO (2) PLY 2 X 4 #2 SPF OR BETTER FASTENED TOGETHER WITH 2 ROWS OF 10d (3" X 0.131") NAILS AT 6" O/C.
- (9) MAINTAIN A MINIMUM 3/4" LUMBER EDGE DISTANCE WHEN NAILING. NAIL SPACING SHOULD APPROXIMATE A MINIMUM 1-3/4" O/C OR MORE UNLESS NOTED OTHERWISE. ALL CONSTRUCTION TO CONFORM TO ONTARIO BUILDING CODE (CURRENT ADDITION) AT ALL TIMES.

NOTES:

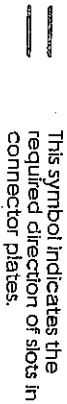
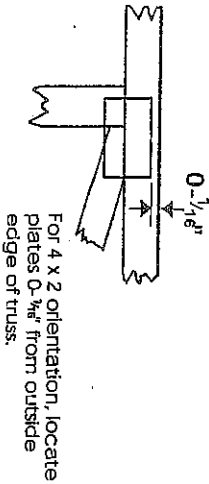
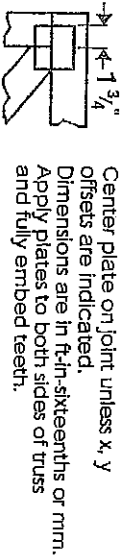
- (10) 48" O/C (MAXIMUM POST SPACING).
- (11) ROOF LIVE LOAD = 34.8 PSF (MAX.)
- (12) ROOF DEAD LOAD = 10.0 PSF (MAX.)
- (13) PART 9 APPLICATION ONLY (ONTARIO BUILDING CODE)
- (14) PART 4 APPLICATION ONLY (ONTARIO BUILDING CODE) WITH APPROVED REVIEW BY LICENSED PROFESSIONAL ENGINEER.
- (15) BASE TRUSS SPACING (24" O/C MAX.)
- (16) ALL PRE-ENGINEERED BASE TRUSS COMPONENTS TO BE SEALED BY LICENSED PROFESSIONAL ENGINEER AND THIS DETAIL TO BE VERIFIED AND APPROVED BY SAME WHEN RIDGE BOARD LENGTH EXCEEDS 12'-0".
- (17) ALL BASE TRUSSES: P = 4 (4/12) - MINIMUM.
- (18) ALL VALLEY RAFTERS: P = 4 (4/12) - MINIMUM.



DWG NO TAM 6305.14
STRUCTURAL
COMPONENT ONLY

Symbols

PLATE LOCATION AND ORIENTATION



* Plate location details available in Mitek software or upon request.

PLATE SIZE

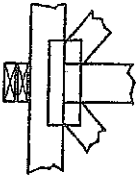
4 X 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

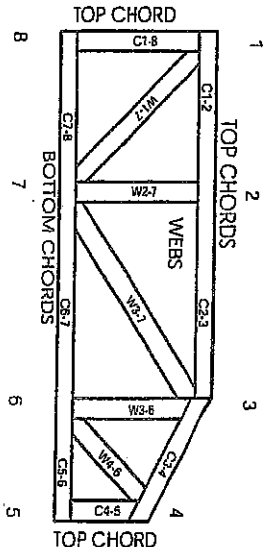
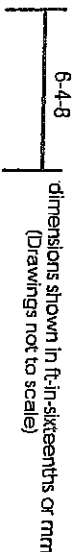


BEARING



Industry Standards:
TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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General Safety Notes

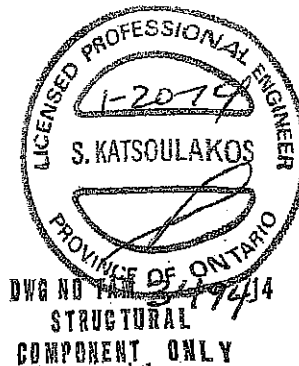
Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative 1, 1, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and warps at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

Micro City Engineering Services Inc.
(BCIN: 26064; FIRM BCIN: 29991)

RR #1, Po Box 61
Glencoe, Ontario
N0L 1M0

(519) 287 - 2242; Fax: (519) 287 - 5750 (Call)



Responsibilities:

Micro City Engineering Services is responsible for the design of trusses as individual components.

It is the responsibilities of others to ascertain that the design loads utilized on this (these) drawing(s) meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdiction over such decisions.

All dimensions are to be verified by the owner, contractor, architect, or other authority having input over such decisions prior to truss component manufacture. At no time shall Micro City Engineering Services Inc. or its employees be responsible for dimension errors.

Micro City Engineering Services Inc. bears no responsibility for the erection of any truss components. Persons erecting truss components are cautioned to seek professional advice regarding temporary and permanent bracing systems and to be **totally** familiar with all aspects of truss erection prior to proceeding on **any** truss component erection job. Any bracing shown on Micro City Engineering Services Inc. or Tamarack Roof Trusses Inc. sealed or unsealed truss component drawings is specified for the single truss component in question and is identified as an integral part of the design for that particular truss component but is **not** meant to represent the only required bracing for that particular truss component when installed as a component in a series of truss components in a roof truss system.

It is the truss manufacturer's responsibility to ensure that trusses are manufactured in accordance with Micro City Engineering Services Inc. specifications outlined below:

SPECIFICATIONS:

Truss components sealed by Micro City Engineering Services Inc. must conform to the relevant sections of the current Building Code of Ontario and Canada (Part 4 or Part 9) or the current Farm Building Code of Canada in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the Truss Plate Institute of Canada (TPIC). All unit lumber and nailing stresses identified on truss component design drawings and/or used in the design of individual truss components shall conform to the current CSA Wood Design standard identified in the current Building Code and TPIC Design Standards.

The lumber used to manufacture any truss component is to conform to the specified size and grade identified on the truss drawing.

The lumber used in the manufacture of any truss component is not to exceed 19% during its service use unless specifically noted on the truss drawing.

The lumber used in the manufacture of any truss component is not to be treated with any chemicals during its service life unless specifically noted on the truss drawing.

Connector plates shall be applied to both faces of the truss component at each joint and shall be positioned exactly as specified.

The top chord of any truss component is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the sealed truss component drawing but not exceeding 24" o/c (Part 9 design) and not exceeding 48" o/c (Part 4 or Agricultural design).

When a truss component is to be installed with no rigid ceiling attached directly to the bottom chord, then the bottom chord is to be laterally braced at intervals not exceeding 3m (or 10'-0").

All sealed or unsealed truss component drawings provided by Micro City Engineering Services Inc. Or Tamarack Roof Trusses Inc. should be read in conjunction with the following:

Warning-Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473C rev 10-'08 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer - not the truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpik.ca and BCSI Building Component Safety Information available from the Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA, 22314.