

ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:
LUS24 - (O)
HGUS26-2 - (XX)

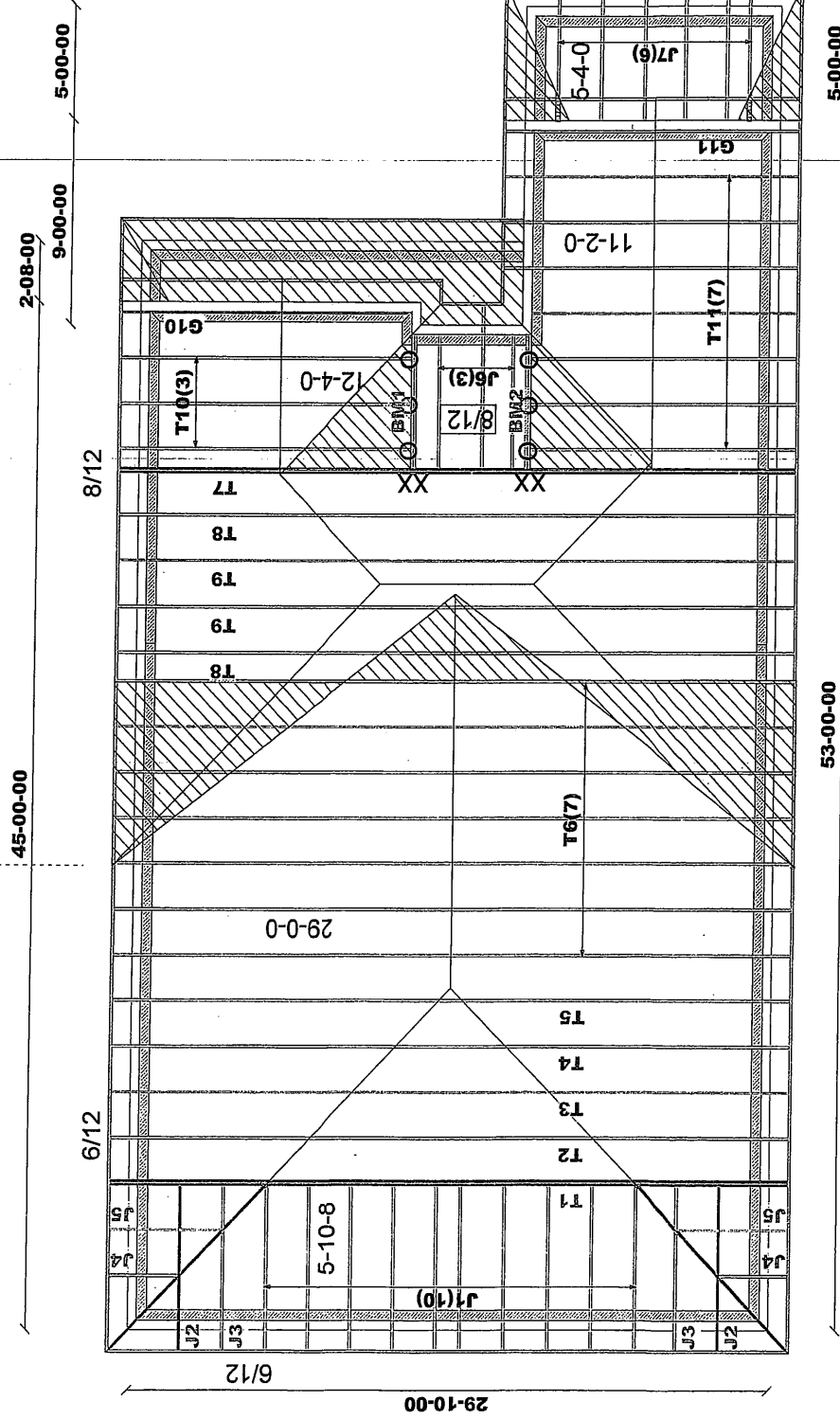
DENOTES
CONV.
FRAMING

DESIGN CONFORMS
WITH O.B.C. 2012 PART 9
DESIGN LOADS:
SNOW LOAD 25.6 PSF
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

T - 170226

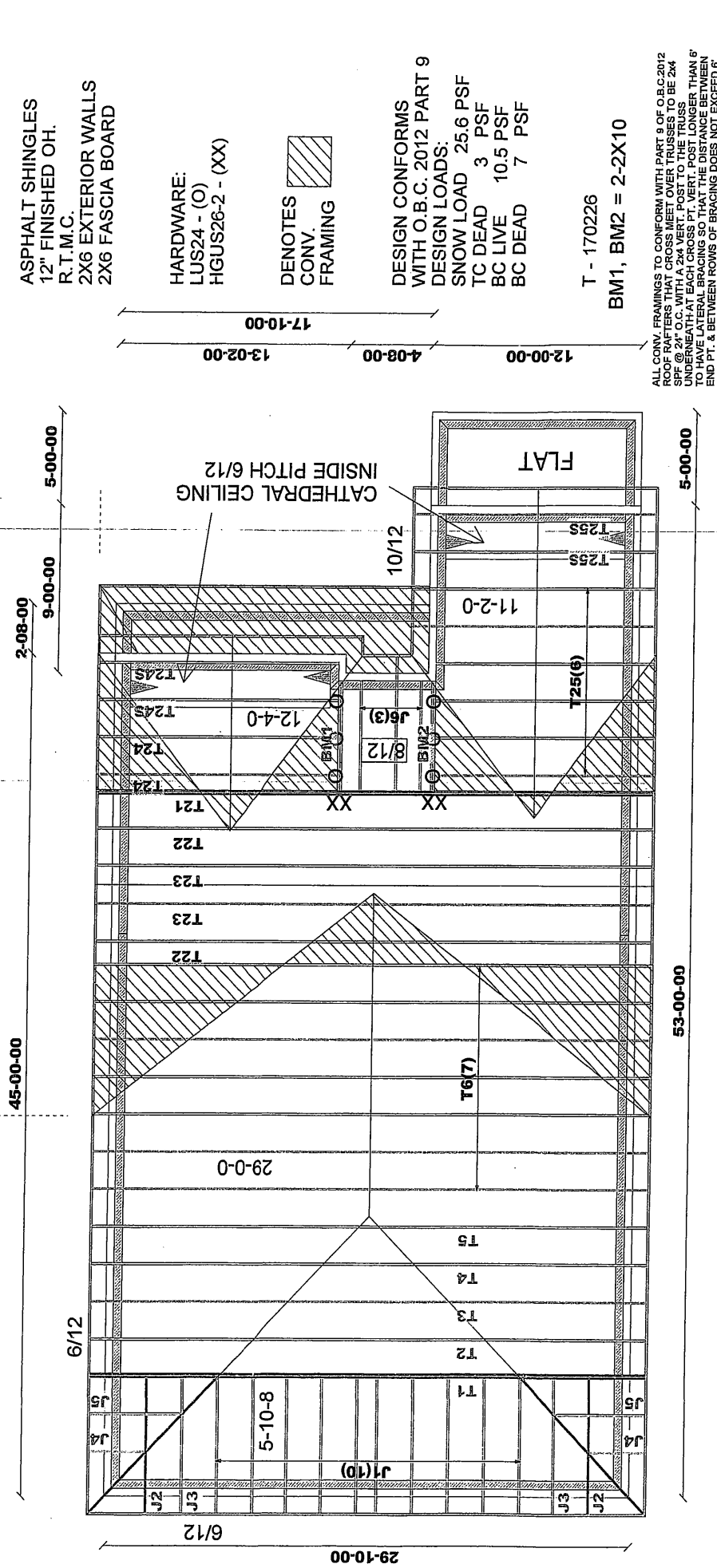
BM1, BM2 = 2-2X10

ALL CONV. FRAMINGS TO CONFORM WITH PART 9 OF O.B.C. 2012
ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2X4
SPF @ 24" O.C. WITH A 2X4 VERT. POST TO THE TRUSS
UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6'
TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN
END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'



m10/619

TAMARACK TRUSS & SCHEMATIC ATA UNIFORM BOARD	Job Track: 43954	Builder / Location:	Model / Elevation:
	Layout ID: 280012	GREENPARK HOMES / WATERDOWN	FLEMING 1 / 1
Plan Log: 91552	Project: RUSSELL GARDENS PHASE 2	THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.	
	Date: 3/27/2017	Designer: AC	



10/6/19

TAMARACK ROOF TRUSSES INC. 1000 10TH AVE. S.W. VANCOUVER, B.C. V6P 6E6 TEL: 604-271-1111 FAX: 604-271-1112 WWW.TAMARACKTRUSSES.COM	Job Track: 43954 Layout ID: 280013 Plan Log: 91552	Builder / Location: GREENPARK HOMES / WATERDOWN	Model / Elevation: FLEMING 1 / 2	THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE. Mitek ver 7.5.0
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ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

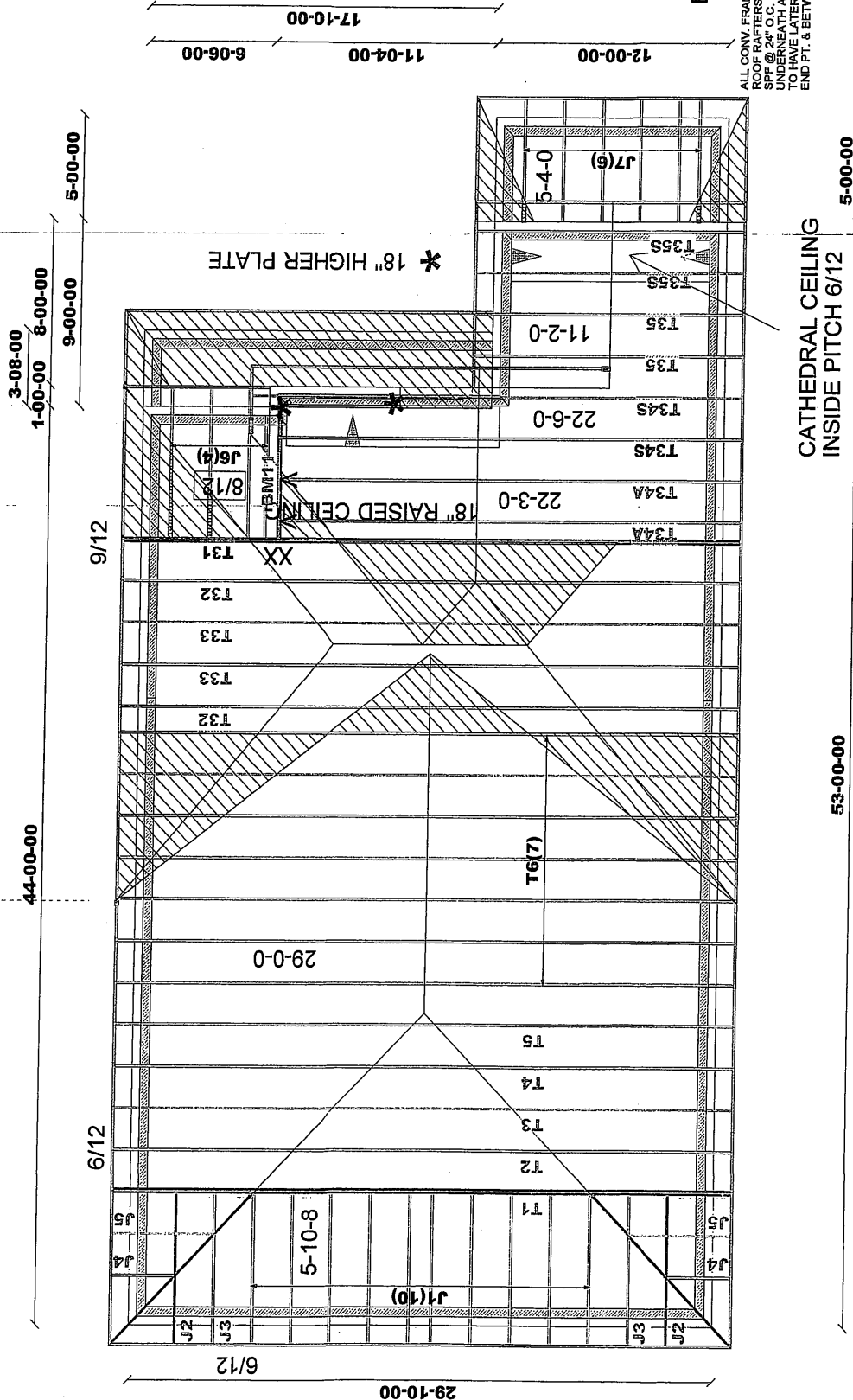
HARDWARE:
LUS24 - (O)
HGUS26-2 - (XX)

DENOTES
CONV.
FRAMING

DESIGN CONFORMS
WITH O.B.C. 2012 PART 9
DESIGN LOADS:
SNOW LOAD 25.6 PSF
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

T - 170226
BM11 = 2-2X10

ALL CONV. FRAMINGS TO CONFORM WITH PART 9 OF O.B.C. 2012
ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2X4
SPF @ 24" O.C. WITH A 2X4 VERT. POST TO THE TRUSS
UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6'
TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN
END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'



CATHEDRAL CEILING
INSIDE PITCH 6/12

m10/1019

TAMARACK CONSTRUCTION INC. A LUMBER GROUP	Job Track: 43954	Builder / Location: GREENPARK HOMES / WATERDOWN	Model / Elevation: FLEMING 1 / 3
	Layout ID: 280014	Project: RUSSELL GARDENS PHASE 2	THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.
Plan Log: 91552	Date: 3/28/2017	Designer: AC	Mitek ver. 7.5.0

DATE	05/09/17
SALES REP	Mario

Delivery Shiplist

JOB TRACK:43954	LAYOUT ID: 280012	LOCATION: WATERDOWN
BUILDER: GREENPARK - RUSSELL GARDENS PH	SUB-BUILDER:	
MODEL: FLEMING 1	ELEVATION: 1	



ROOF TRUSSES

PROFILE	QTY	MARK	PITCH	SPAN	TRUSS	LUMBER	OVERHANG	HEEL HEIGHT	LBS.	BUNDLE #	REMARKS
	PLY	TYPE	TC BC		HEIGHT	TOP BOT	LEFT RIGHT	LEFT RIGHT	BFT.		
	1	T1	0.00	29-00-00	04-01-04	2 X 4	01-03-08	01-02-00	232.18		
	2 Ply	HIP GIRDER	6.00						150.66		
	1	T2	6.00	29-00-00	05-01-04	2 X 4	01-03-08	01-02-00	112.13		
		HIP	0.00						70.83		
	1	T3	6.00	29-00-00	06-01-04	2 X 4	01-03-08	01-02-00	120.87		
		HIP	0.00						77.34		
	1	T4	6.00	29-00-00	07-01-04	2 X 4	01-03-08	01-02-00	117.97		
		HIP	0.00						73.83		
	1	T5	6.00	29-00-00	08-01-04	2 X 4	01-03-08	01-02-00	123.94		
		HIP	0.00						78.67		
	7	T6	6.00	29-00-00	08-05-00	2 X 4	01-03-08	01-02-00	792.61		
		COMMON	0.00						492.31		
	1	T7	8.00	29-00-00	05-03-13	2 X 4	01-03-08	01-04-13	291.18		
	2 Ply	HIP GIRDER	0.00						181.00		
	2	T8	8.00	29-00-00	06-07-13	2 X 4	01-03-08	01-04-13	243.26		
		HIP	0.00						156.34		
	2	T9	8.00	29-00-00	07-11-13	2 X 4	01-03-08	01-04-13	267.42		
		HIP	0.00						169.34		
	3	T10	8.00	12-04-00	05-06-02	2 X 4	01-03-08	01-04-13	151.83		
		COMMON	0.00						95.52		
	1	G10	8.00	12-04-00	05-06-02	2 X 4	01-03-08	01-04-13	53.93		
		COMMON	0.00						35.84		
	7	T11	8.00	11-02-00	05-01-08	2 X 4	01-03-08	01-04-13	325.01		
		COMMON	0.00						208.81		
	1	G11	8.00	11-02-00	05-01-08	2 X 4	01-03-08	01-04-13	48.64		
		COMMON	0.00						31.83		
	10	J1	6.00	05-10-08	04-01-04	2 X 4	01-03-08	01-02-00	167.90		
		JACK-OPEN	0.00					04-01-04	106.70		
	2	J2	6.00	05-10-08	02-00-12	2 X 4	01-03-08	01-02-00	23.16		
		JACK-OPEN	0.00					00-03-08	14.66		
	2	J3	6.00	05-10-08	03-00-12	2 X 4	01-03-08	01-02-00	28.26		
		JACK-OPEN	0.00					00-03-08	17.34		
	2	J4	6.00	01-10-08	02-00-12	2 X 4	01-03-08	01-02-00	14.04		
		JACK-OPEN	0.00					00-03-08	9.34		
	2	J5	6.00	01-10-08	03-00-12	2 X 4	01-03-08	01-02-00	19.14		
		JACK-OPEN	0.00					00-04-11	12.00		

ROOF TRUSS SPACING:24.0 IN. O.C. (TYP.)



Delivery Shiplist

DATE	05/09/17
SALES REP	Mario

JOB TRACK:43954	LAYOUT ID: 280012	LOCATION: WATERDOWN
BUILDER: GREENPARK - RUSSELL GARDENS PH	SUB-BUILDER:	
MODEL: FLEMING 1	ELEVATION: 1	

ROOF TRUSSES

PROFILE	QTY	PLY	MARK	TYPE	PITCH	SPAN	TRUSS	LUMBER		OVERHANG	HEEL HEIGHT	LBS.	BUNDLE #	REMARKS
								TOP	BOT					
	3		J6	JACK-OPEN	8.00	05-10-08	05-03-13	2 X 4	2 X 4	01-03-08	01-04-13	53.49		
	6		J7	JACK-OPEN	4.00	05-04-00	02-01-04	2 X 4	2 X 4	01-03-08	00-03-15	84.90		

TOTAL # TRUSS= 58.00 TOTAL BFT OF ALL TRUSSES= 2070.35 BFT. TOTAL WEIGHT OF ALL TRUSSES= 3271.86 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH
6	Hangers	LUS24	
2	Hangers	HGUS26-2	

TOTAL # ITEMS= 8.00

DATE	05/09/17
SALES REP	Mario

Delivery Shipplit

JOB TRACK:43954	LAYOUT ID: 280013	LOCATION: WATERDOWN
BUILDER: GREENPARK - RUSSELL GARDENS PH	SUB-BUILDER:	
MODEL: FLEMING 1	ELEVATION: 2	



ROOF TRUSSES

PROFILE	QTY	MARK	PITCH	SPAN	TRUSS	LUMBER	OVERHANG	HEEL HEIGHT	LBS.	BUNDLE #	REMARKS
	PLY	TYPE	TC BC		HEIGHT	TOP BOT	LEFT RIGHT	LEFT RIGHT			
	1	T1	0.00	29-00-00	04-01-04	2 X 4	01-03-08	01-02-00	232.18		
	2 Ply	HIP GIRDER	6.00						150.66		
	1	T2	0.00	29-00-00	05-01-04	2 X 4	01-03-08	01-02-00	112.13		
		HIP	6.00						70.83		
	1	T3	0.00	29-00-00	06-01-04	2 X 4	01-03-08	01-02-00	120.87		
		HIP	6.00						77.34		
	1	T4	0.00	29-00-00	07-01-04	2 X 4	01-03-08	01-02-00	117.97		
		HIP	6.00						73.83		
	1	T5	0.00	29-00-00	08-01-04	2 X 4	01-03-08	01-02-00	123.94		
		HIP	6.00						78.67		
	7	T6	0.00	29-00-00	08-05-00	2 X 4	01-03-08	01-02-00	792.61		
		COMMON	6.00						492.31		
	1	T21	0.00	29-00-00	05-03-13	2 X 4	01-03-08	05-03-13	297.12		
	2 Ply	FLAT GIRDER	0.00						183.00		
	2	T22	0.00	29-00-00	06-07-13	2 X 4	01-03-08	06-07-13	255.60		
		FLAT	0.00						162.34		
	2	T23	0.00	29-00-00	07-11-13	2 X 4	01-03-08	07-11-13	300.70		
		FLAT	0.00						186.66		
	2	T24	0.00	12-04-00	06-09-05	2 X 4	01-03-08	01-07-10	118.80		
		COMMON	10.00						76.34		
	2	T24S	0.00	12-04-00	06-09-05	2 X 4	01-03-08	01-07-10	112.98		
		SCISSORS	6.00						72.66		
	6	T25	0.00	11-02-00	06-03-07	2 X 4	01-03-08	01-07-10	298.20		
		COMMON	10.00						198.00		
	2	T25S	0.00	11-02-00	06-03-07	2 X 4	01-03-08	01-07-10	98.88		
		SCISSOR	6.00						64.00		
	10	J1	0.00	05-10-08	04-01-04	2 X 4	01-03-08	01-02-00	167.90		
		JACK-OPEN	6.00						106.70		
	2	J2	0.00	05-10-08	02-00-12	2 X 4	01-03-08	01-02-00	23.16		
		JACK-OPEN	6.00						14.66		
	2	J3	0.00	05-10-08	03-00-12	2 X 4	01-03-08	01-02-00	28.26		
		JACK-OPEN	6.00						17.34		
	2	J4	0.00	01-10-08	02-00-12	2 X 4	01-03-08	01-02-00	14.04		
		JACK-OPEN	6.00						9.34		
	2	J5	0.00	01-10-08	03-00-12	2 X 4	01-03-08	01-02-00	19.14		
		JACK-OPEN	6.00						12.00		

ROOF TRUSS SPACING:24.0 IN. O.C. (TYP.)



Delivery Shiplist

DATE	05/09/17
SALES REP	Mario

JOB TRACK:43954	LAYOUT ID: 280013	LOCATION: WATERDOWN
BUILDER: GREENPARK - RUSSELL GARDENS PH	SUB-BUILDER:	
MODEL: FLEMING 1	ELEVATION: 2	

ROOF TRUSSES

PROFILE	QTY	PLY	MARK	TYPE	PITCH	SPAN	TRUSS	LUMBER		OVERHANG	HEEL HEIGHT	LBS.	BUNDLE #	LOAD BY: REMARKS
								TOP	BOT					
7	3		J6	JACK-OPEN	8.00	05-10-08	05-03-13	2 X 4	2 X 4	01-03-08	01-04-13	53.49		
					0.00					00-00-00	05-03-13	32.01		

ROOF TRUSS SPACING:24.0 IN. O.C. (TYP.)

TOTAL # TRUSS=52.00
TOTAL BFT OF ALL TRUSSES=

2078.69 BFT. TOTAL WEIGHT OF ALL TRUSSES= 3287.97 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH
6	Hangers	LUS24	
2	Hangers	HGUS26-2	

TOTAL # ITEMS= 8.00

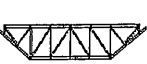
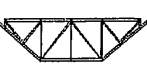
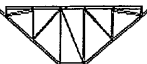
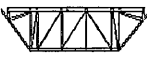
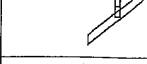
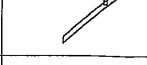


Delivery Shiplist

DATE	05/09/17
SALES REP	Mario

JOB TRACK:43954	LAYOUT ID: 280014	LOCATION: WATERDOWN
BUILDER: GREENPARK - RUSSELL GARDENS PH	SUB-BUILDER:	
MODEL: FLEMING 1	ELEVATION: 3	

ROOF TRUSSES

PROFILE	QTY	MARK	PITCH	SPAN	TRUSS	LUMBER	OVERHANG	HEEL HEIGHT	LBS.	BUNDLE #	LOAD BY: REMARKS
	PLY	TYPE	BC TC		HEIGHT	TOP BOT	LEFT RIGHT	LEFT RIGHT	BFT.		
	1	T1	0.00	29-00-00	6.00	04-01-04	01-03-08	01-02-00	232.18		
	2 PLY	HIP GIRDER							150.66		
	1	T2	6.00	29-00-00	6.00	05-01-04	01-03-08	01-02-00	112.13		
		HIP	0.00						70.83		
	1	T3	6.00	29-00-00	6.00	06-01-04	01-03-08	01-02-00	120.87		
		HIP	0.00						77.34		
	1	T4	6.00	29-00-00	6.00	07-01-04	01-03-08	01-02-00	117.97		
		HIP	0.00						73.83		
	1	T5	6.00	29-00-00	6.00	08-01-04	01-03-08	01-02-00	123.94		
		HIP	0.00						78.67		
	7	T6	6.00	29-00-00	6.00	08-05-00	01-03-08	01-02-00	792.61		
		COMMON	0.00						492.31		
	1	T31	9.00	29-00-00	9.00	05-03-13	01-03-08	00-00-00	300.34		
	2 PLY	HIP GIRDER	0.00						188.34		
	2	T32	9.00	29-00-00	9.00	06-07-13	01-03-08	01-06-04	261.78		
		HIP	0.00						168.00		
	2	T33	9.00	29-00-00	9.00	07-11-13	01-03-08	01-06-04	268.40		
		HIP	0.00						172.00		
	2	T34A	9.00	22-03-00	9.00	10-08-08	01-03-08	01-06-04	212.14		
		COMMON	0.00						134.34		
	2	T34S	9.00	22-06-00	9.00	10-08-08	01-03-08	00-00-00	231.72		
		ROOF	0.00						149.34		
	2	T35	9.00	11-02-00	9.00	05-08-08	01-03-08	01-06-04	96.10		
		COMMON	0.00						63.34		
	2	T35S	9.00	11-02-00	9.00	05-08-08	01-03-08	01-06-04	97.28		
		SCISSOR	6.00						61.34		
	10	J1	6.00	05-10-08	6.00	04-01-04	01-03-08	01-02-00	167.90		
		JACK-OPEN	0.00						106.70		
	2	J2	6.00	05-10-08	6.00	02-00-12	01-03-08	01-02-00	23.16		
		JACK-OPEN	0.00						14.66		
	2	J3	6.00	05-10-08	6.00	03-00-12	01-03-08	01-02-00	28.26		
		JACK-OPEN	0.00						17.34		
	2	J4	6.00	01-10-08	6.00	02-00-12	01-03-08	01-02-00	14.04		
		JACK-OPEN	0.00						9.34		
	2	J5	6.00	01-10-08	6.00	03-00-12	01-03-08	01-02-00	19.14		
		JACK-OPEN	0.00						12.00		

ROOF TRUSS SPACING:24.0 IN. O.C. (TYP.)

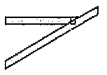


Delivery Shiplist

DATE	05/09/17
SALES REP	Mario

JOB TRACK:43954	LAYOUT ID: 280014	LOCATION: WATERDOWN
BUILDER: GREENPARK - RUSSELL GARDENS PH	SUB-BUILDER:	
MODEL: FLEMING 1	ELEVATION: 3	

ROOF TRUSSES

PROFILE	QTY	PLY	MARK	TYPE	PITCH	SPAN	TRUSS	LUMBER		OVERHANG	HEEL HEIGHT	LBS.	BUNDLE #	LOAD BY: REMARKS
								TOP	BOT					
	4		J6	JACK-OPEN	8.00	05-10-08	05-03-13	2 X 4	2 X 4	01-03-08	00-00-00	42.68		
	6		J7	JACK-OPEN	4.00	05-04-00	02-01-04	2 X 4	2 X 4	01-03-08	00-00-00	84.90		

TOTAL # TRUSS= 55.00

TOTAL BFT OF ALL TRUSSES=

2139.04 BFT. TOTAL WEIGHT OF ALL TRUSSES= 3376.18 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH
2	Hangers	LJS26DS	
1	Hangers	HGUS26-2	

TOTAL # ITEMS= 3.00

JOB NAME
280012

TRUSS NAME
T1

QUANTITY
1

PLY
2

JOB DESC.
43864 TRUSS DESC.

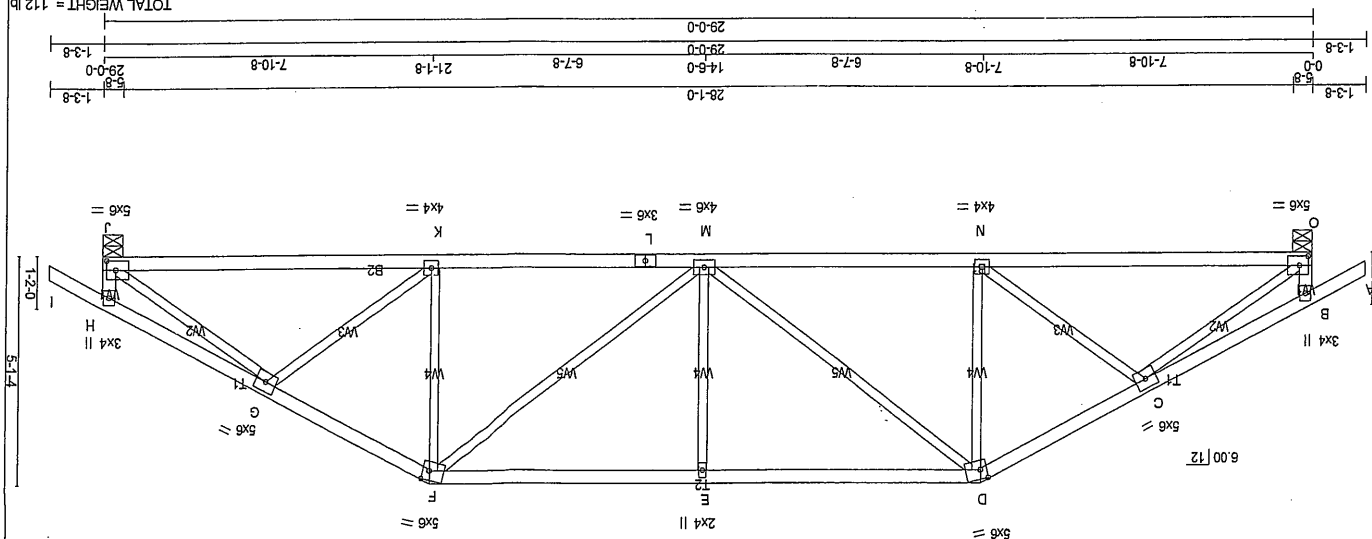
DRWG NO.

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MITTEK Industries, Inc. Wed May 10 00:32:54 2017 Page 2
ID: Cn XbeghZimg3BRHb0z7uzWIOS-7EY57EGQ40xbnNlgqgx4Cmh77rPHpHw15214WV

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 428.7 lbs FACTORED DOWN AT 5-10-8,
101.4 lbs FACTORED DOWN AT 7-11-4, 101.4 lbs
FACTORED DOWN AT 9-11-4, 101.4 lbs
FACTORED DOWN AT 13-11-4, 101.4 lbs
FACTORED DOWN AT 15-0-12, 101.4 lbs
FACTORED DOWN AT 17-0-12, 101.4 lbs
FACTORED DOWN AT 23-1-8 ON TOP CHORD,
AND 746 lbs FACTORED DOWN AT 1-11-4, 69.9
lbs FACTORED DOWN AT 3-11-4, 69.9 lbs
FACTORED DOWN AT 5-11-4, 69.9 lbs
FACTORED DOWN AT 7-11-4, 69.9 lbs
FACTORED DOWN AT 9-11-4, 69.9 lbs
FACTORED DOWN AT 11-11-4, 69.9 lbs
FACTORED DOWN AT 13-11-4, 69.9 lbs
FACTORED DOWN AT 15-0-12, 69.9 lbs
FACTORED DOWN AT 17-0-12, 69.9 lbs
FACTORED DOWN AT 19-0-12, 69.9 lbs
FACTORED DOWN AT 21-0-12, AND 428.7 lbs
FACTORED DOWN AT 23-1-8 ON TOP CHORD,
AND 746 lbs FACTORED DOWN AT 1-11-4, 69.9
lbs FACTORED DOWN AT 3-11-4, 69.9 lbs
FACTORED DOWN AT 5-11-4, 69.9 lbs
FACTORED DOWN AT 7-11-4, 69.9 lbs
FACTORED DOWN AT 9-11-4, 69.9 lbs
FACTORED DOWN AT 11-11-4, 69.9 lbs
FACTORED DOWN AT 13-11-4, 69.9 lbs
FACTORED DOWN AT 15-0-12, 69.9 lbs
FACTORED DOWN AT 17-0-12, 69.9 lbs
FACTORED DOWN AT 19-0-12, 69.9 lbs
FACTORED DOWN AT 21-0-12, AND 746 lbs
FACTORED DOWN AT 23-0-12, AND 69.9 lbs
AC 1-11-4 -40 -75
AD 3-11-4 -40 -70
AE 7-11-4 -40 -70
AF 9-11-4 -40 -70
AG 11-11-4 -40 -70
AH 13-11-4 -40 -70
AJ 15-0-12 -40 -70
AK 17-0-12 -40 -70
AL 21-0-12 -40 -70
AM 25-0-12 -40 -70
AN 27-0-12 -40 -75

FACTORED CONCENTRATED LOADS (LBS)		MAX-		MAX+		FACE		DIR		TYPE	
AN	27.0-12	-40	-75	—	—	BACK	VERT	TOTAL	TOTAL	TOTAL	TOTAL
AM	25.0-12	-40	-70	—	—	BACK	VERT	TOTAL	TOTAL	TOTAL	TOTAL
AL	21.0-12	-40	-70	—	—	BACK	VERT	TOTAL	TOTAL	TOTAL	TOTAL
AJ	17.0-12	-40	-70	—	—	BACK	VERT	TOTAL	TOTAL	TOTAL	TOTAL
AI	15.0-12	-40	-70	—	—	BACK	VERT	TOTAL	TOTAL	TOTAL	TOTAL
AH	13.1-14	-40	-70	—	—	BACK	VERT	TOTAL	TOTAL	TOTAL	TOTAL
AG	11.1-14	-40	-70	—	—	BACK	VERT	TOTAL	TOTAL	TOTAL	TOTAL
AF	9.1-14	-40	-70	—	—	BACK	VERT	TOTAL	TOTAL	TOTAL	TOTAL
AE	7.1-14	-40	-70	—	—	BACK	VERT	TOTAL	TOTAL	TOTAL	TOTAL
AD	3.1-14	-40	-70	—	—	BACK	VERT	TOTAL	TOTAL	TOTAL	TOTAL
AC	1.1-14	-40	-75	—	—	BACK	VERT	TOTAL	TOTAL	TOTAL	TOTAL
AB	2.10-12	-101	-101	—	—	BACK	VERT	TOTAL	TOTAL	TOTAL	TOTAL
Z	17.0-12	-101	-101	—	—	BACK	VERT	TOTAL	TOTAL	TOTAL	TOTAL
Y	15.0-12	-101	-101	—	—	BACK	VERT	TOTAL	TOTAL	TOTAL	TOTAL
X	13.1-14	-101	-101	—	—	BACK	VERT	TOTAL	TOTAL	TOTAL	TOTAL
W	11.1-14	-101	-101	—	—	BACK	VERT	TOTAL	TOTAL	TOTAL	TOTAL
V	9.1-14	-101	-101	—	—	BACK	VERT	TOTAL	TOTAL	TOTAL	TOTAL
U	7.1-14	-101	-101	—	—	BACK	VERT	TOTAL	TOTAL	TOTAL	TOTAL
S	5.1-14	-40	-70	—	—	BACK	VERT	TOTAL	TOTAL	TOTAL	TOTAL
O	19.0-12	-40	-70	—	—	BACK	VERT	TOTAL	TOTAL	TOTAL	TOTAL
N	23.0-12	-40	-70	—	—	BACK	VERT	TOTAL	TOTAL	TOTAL	TOTAL
I	23.1-6	-429	-429	—	—	BACK	VERT	TOTAL	TOTAL	TOTAL	TOTAL
H	19.0-12	-101	-101	—	—	BACK	VERT	TOTAL	TOTAL	TOTAL	TOTAL
JT	5.10-8	-429	-429	—	—	BACK	VERT	TOTAL	TOTAL	TOTAL	TOTAL
LOC	LC1	MAX-	MAX+	FACE	DIR	TYPE					

[illegible]

BRACING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S).
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.71 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY
APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

[illegible]

(65% OF 31.3 P.S.F. G.S.F. PLUS 8.4 P.S.F. SPECIFIED
 RAIN LOAD) EQUIVALENT 25.6 P.S.F.

ALLOWABLE DEF'L (L) =	L/360 (0.97")
ALLOWABLE DEF'L (L) =	L/999 (0.18")
CALCULATED VERT. DEF'L (L) =	L/790 (0.27")
ALLOWABLE DEF'L (L) =	L/360 (0.97")

C/S: TC=0.60 (D-E:1), BC=0.65 (M-N:2), WB=0.92
 (C-O:1), SSI=0.27 (D-E:1)
 DOT LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10
 COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER'S CONTROL
 THE TRUSS MANUFACTURING PLANT .

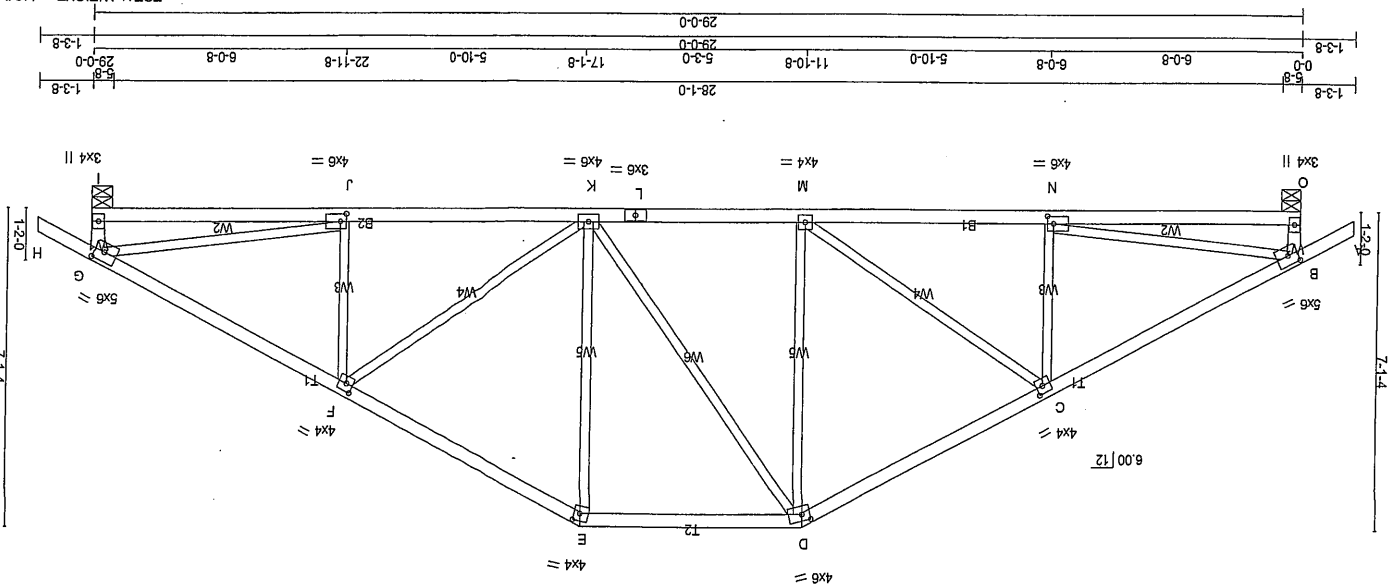
NAIL VALUES	SECTION	SHEAR	PLATE GIRD (DRY)	(PSI)	(PL)	MAX MIN	MT20
1656	2284	1657	822	1656	822	1656	1656

PLATE PLACEMENT TOL. = 0.250 inches
 PLATE ROTATION TOL. = 5.0 Deg.

JSI GIRP = 0.89 (N) (NPUT = 0.99)
 JSI METAL= 0.60 (G) (INPUT = 1.00)



OWG NO. TAM 2136-17
STRUCTURAL
COMPONENT ONLY



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

DESIGN CRITERIA

TOTAL WEIGHT = 118 lb

SPECIFIED LOADS:
 TOP CH. LL = 25.6 PSF
 DL = 3.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 46.1 PSF
 SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCS 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCB 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F., G.S.L. PLUS 8.4 P.S.F.,
RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED
ROOF LIVE LOAD

ALLOWABLE DEFLECTION = $L/360$ (0.97")
CALCULATED VERT. DEFLECTION = $L/999$ (0.12")
ALLOWABLE DEFLECTION = $L/360$ (0.97")
CALCULATED VERT. DEFLECTION = $L/999$ (0.19")

CSI: TC=0.49 (B-C:1), BC=0.48 (M-N:2), WB=0.5 (C-M:1), SSI=0.22 (F-G:1)

COMP=1.10 SHEAR=1.10 TENS=1.10
COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

MAIL VALUES	PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)

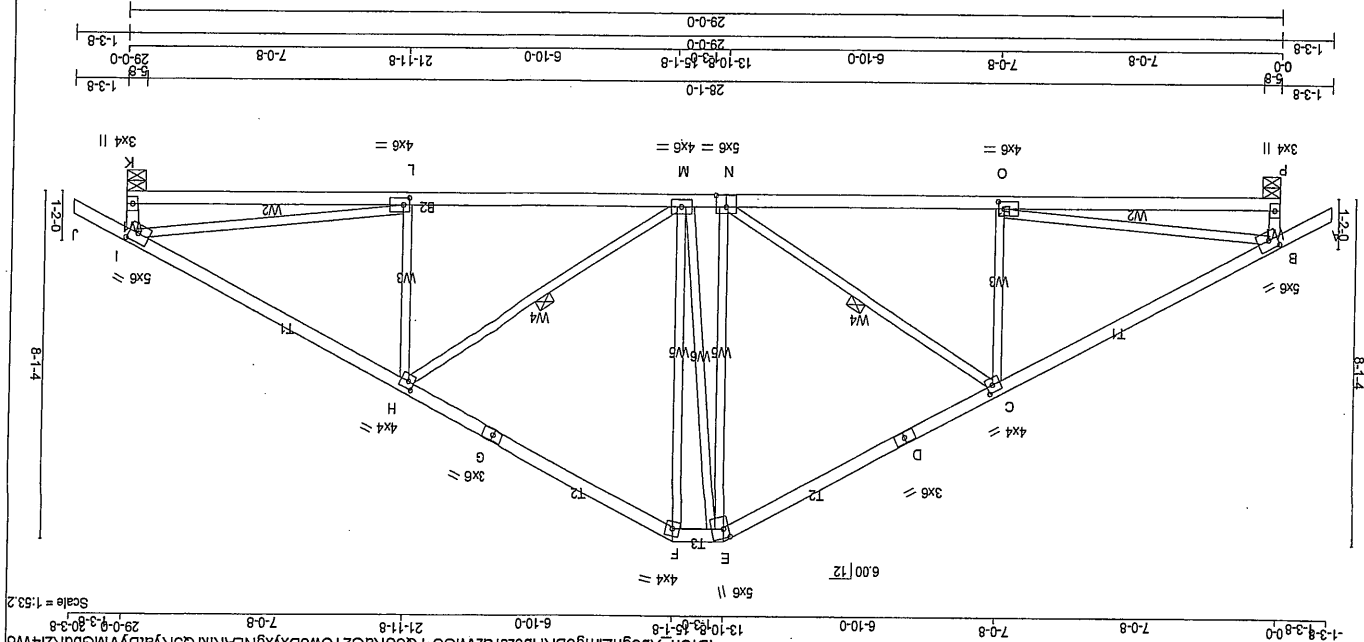
MAX MIN	MAX MIN	MAX MIN	MAX MIN	MAX MIN
6 18	354	1667	822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

DATE ROTATION TOL. = 3.0 Deg.

DWD NO. TAM 21397-17
STRUCTURAL
COMPONENT ONLY

[illegible][illegible]

[illegible]

NUMBER	No.2	No.2	No.2	No.2	No.2	No.2	No.2	No.2
W	5.0	6.0	2.50	2.75	X			
LEN	5.0	4.0	2.00	1.75				
6.0	5.0	4.0	2.50	1.50				
4.0	3.0	4.0						
6.0	4.0	6.0						
5.0	5.0	6.0						
4.0	4.0	6.0						
6.0	5.0	2.00	1.75					
5.0	4.0	2.50						
4.0	3.0	2.00						
6.0	4.0	1.75						
5.0	6.0	3.00						
4.0	5.0	2.00						
3.0	4.0	1.75						

[illegible]

OWNS NO. TAM 21398-17
STRUCTURAL
COMPONENT ONLY

USI GRIP = 0.88 (N) (INPUT = 0.90)
USI METAL = 0.63 (B) (INPUT = 1.00)

PLATE ROTATION TOL. = 5.0 Deg.

PLATE PLACEMENT TOL. = 0.250 inches

	MAX MIN	MAX MIN	MAX MIN	MAX MIN	MT20
	618	354	1667	822	2284 1656

NAIL VALUES

RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

THE UNIVERSITY OF CHICAGO PRESS

COMPANION LIVE LOAD FACTOR = 0.50

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

CSI: TC=0.69 (B-C:1), BC=0.59 (N-O:2), WB=0.48

CALCULATED VERT. DEF. (LL) = 1/999 (0.14")

WIND LOAD/ EXPOSED SURF.: SPECIFIED
ROOF LIVE LOAD

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F.

-PART 9 OF OBC 2012, BCB 2012, ABC 2014
-CSA 086-09

OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

SLOPE OF 6:00/12

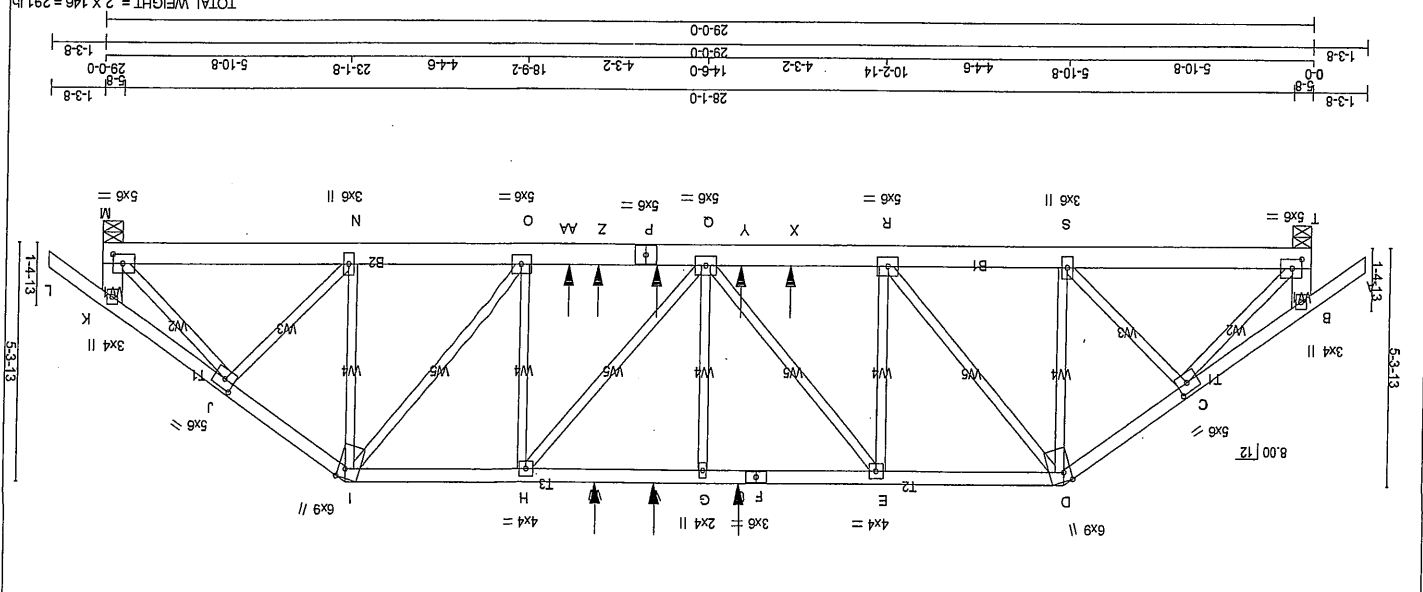
LOADING IN FLAT SECTION BASED ON A

TOTAL LOAD = 46.1 PSF
SPACING = 24.0 IN./C.

DL =	3.0	PSF
LT =	10.5	PSF
DL =	7.0	PSF

SPECIFIED LOADS:
TOP CH. 11 = 25.6 BSE

DESIGN CRITERIA	
TOTAL WEIGHT = 1	



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED GROSS REACTION

FACTORED GROSS REACTION

FACTORED GROSS REACTION

FACTORED GROSS REACTION

FACTORED GROSS REACTION

FACTORED GROSS REACTION

FACTORED GROSS REACTION

FACTORED GROSS REACTION

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FACTORED GROSS REACTION

FACTORED GROSS REACTION

FACTORED GROSS REACTION

FACTORED GROSS REACTION

FACTORED GROSS REACTION

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
280012	T7	1	2			

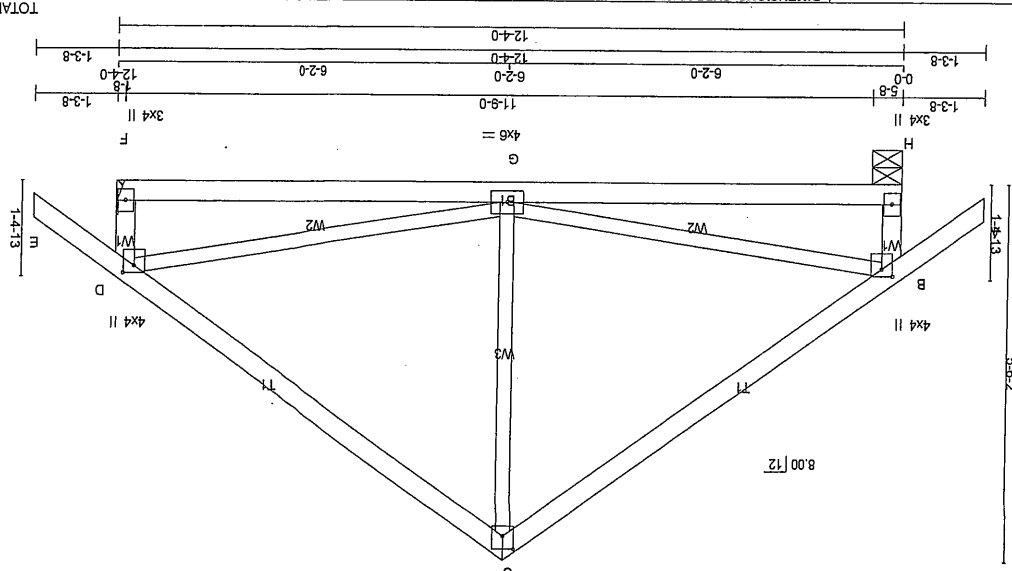
Tamarack Roof Truss, Burlington

ID: Cn XbeqhZimg3BRHb0zst7uzVWIOS-ydgsXWthEH3t65X7E4SPsVH5XVK3JIK6K078Azt4W6
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Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.
HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 101.4 lbs FACTORED DOWN AT
13-7-12, AND 101.4 lbs FACTORED DOWN AT
17-0-4 ON TOP CHORD, AND 1507.0 lbs
FACTORED DOWN AT 12-5-8, 71.7 lbs
FACTORED DOWN AT 13-7-12, 71.7 lbs
FACTORED DOWN AT 15-7-12, AND 71.7 lbs
FACTORED DOWN AT 17-0-4, AND 1406.0 lbs
FACTORED DOWN AT 17-8-8 ON BOTTOM
CHORD. DESIGN FOR UNSPECIFIED
CONNECTION(S) IS DELEGATED TO THE
BUILDING DESIGNER.



DRWG NO. 280012
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 3 X 51 = 152 lb

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH, LL = 25.6 PSF
 BOT CH, LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 46.1 PSF

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010
 THIS DESIGN COMPLETES WITH:
 - PART 9 OF OBC 2012, BCB 2012, ABC 2014
 - CSA 086-09
 - TPC 2011

ALLOWABLE DEFLECTION (LL) = L/360 (0.41")
 ALLOWABLE DEFLECTION (LL) = L/999 (0.04")
 ALLOWABLE DEFLECTION (LL) = L/999 (0.07")
 CALCULATED VERT. DEFLECTION (LL) = L/999 (0.07")
 CS: TC=0.42 (B-C:1), BC=0.31 (G-H:3), WB=0.10 (D-G:1), SS=0.17 (B-C:1)
 DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10
 COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP (DRY) SHEAR SECTION
 MAX MIN MAX MIN
 (PS) (PL)
 MT20 618 354 1667 822 2284 1656
 PLATE PLACEMENT TOL. = 0.250 inches
 PLATE ROTATION TOL. = 5.0 Deg

JSI METAL = 0.17 (D) (INPUT = 1.00)
 JSI GRIP = 0.60 (G) (INPUT = 0.90)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
 FACTORED REACTION
 GROSS REACTION
 DOWN HORIZ
 UP HORIZ
 BRG IN-SX
 BRG IN-SX
 INPUT REAR

UNFACTORED REACTIONS
 1ST CASE
 MAX MIN COMPONENT REACTIONS
 LIVE PERM LIVE WIND DEAD SOIL

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H
 MAX UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
 ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

CHORDS
 FACTORED
 FORCE
 MEMB.
 FR TO
 FROM TO
 (PL) CS (LC) UNBRAC (LBS)
 MAX FACTORED VERT. LOAD LC1 MAX MEMB. FORCE MAX CS (LC)

TOTAL LOAD CASES: (4)
 CHORDS
 FACTORED
 FORCE
 MEMB.
 FR TO
 FROM TO
 (PL) CS (LC) UNBRAC (LBS)
 MAX FACTORED VERT. LOAD LC1 MAX MEMB. FORCE MAX CS (LC)

CHORDS
 FACTORED
 FORCE
 MEMB.
 FR TO
 FROM TO
 (PL) CS (LC) UNBRAC (LBS)
 MAX FACTORED VERT. LOAD LC1 MAX MEMB. FORCE MAX CS (LC)



DWG NO. T10-2403-17
 STRUCTURAL
 COMPONENT ONLY



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH	LL	25.6	PSF
BOT CH	LL	3.0	PSF
	DL	10.5	PSF
	DL	7.0	PSF
TOTAL LOAD	=	46.1	PSF

IN GC

SPACING = 24.0

THIS TRUSS IS DESIGNED FOR PERMANENT AND SMALL BUILDING REQUIREMENTS OF PART 9, NBC C 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2012, BBC 2012, ABC 2014
- CSA 086-09
- TPI C 2011

(55 % OF 31.3 P.S.F., G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUATES 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11 (H+1), BC=0.03 (N+0.2), WB=0.06 (E+M-1), SS=0.08 (H-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

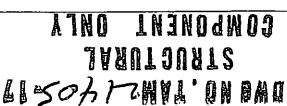
NAIL VALUES

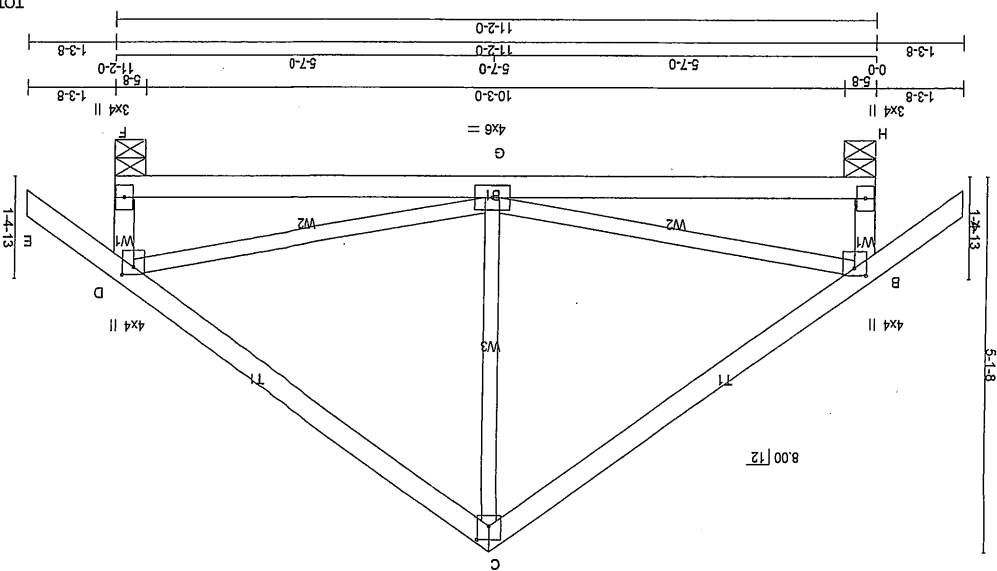
PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)
MT20	618	354
	MAX MIN	1667 822
	MAX MIN	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (B) (INPUT = 0.90)
JSI METAL= 0.05 (C) (INPUT = 1.00)





TOTAL WEIGHT = 7 X 46 = 325 lb

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 25.6 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 46.1 PSF
 SPACING = 24.0 IN. C/C

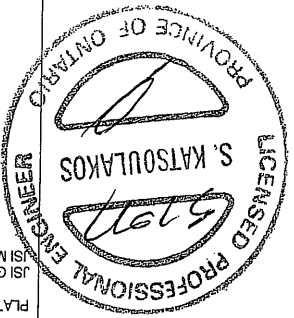
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010
 THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TFC 2011

ALLOWABLE DEFLECTION (L) = L/360 (0.37")
 ALLOWABLE DEFLECTION (LL) = L/999 (0.03")
 CALCULATED VERT. DEFLECTION (LL) = L/999 (0.03")
 ALLOWABLE DEFLECTION (LL) = L/360 (0.37")
 CALCULATED VERT. DEFLECTION (LL) = L/999 (0.03")
 (55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED

COMPANION LIVE LOAD FACTOR = 0.50
 RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.
 NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION
 MAX MIN MAX MIN
 (PSI) (PLI)
 MT20 618 354 1667 822 2284 1656
 PLATE PLACEMENT TOL. = 0.250 inches
 PLATE ROTATION TOL. = 5.0 Deg

JSI GRIP = 0.54 (G) (INPUT = 0.90)
 JSI METAL = 0.15 (D) (INPUT = 1.00)

DWG NO. TAM 21404-17
 STRUCTURAL COMPONENT ONLY



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	GROSS REACTION	MAXIMUM FACTORED	INPUT	RECORD
DESCR.	SPF	SPF	SPF	BRG	BRG
LUMBER	No.2	No.2	No.2	IN-SX	IN-SX
CHORDS	2x4	2x4	2x4	IN-SX	IN-SX
N.L.G. RULES	2x4	2x4	2x4	IN-SX	IN-SX
EXCEPT	DRY	DRY	DRY	IN-SX	IN-SX
ALL WEBS	2x3	2x4	2x4	IN-SX	IN-SX
DRY, SEASONED LUMBER.	DRY	DRY	DRY	IN-SX	IN-SX

1ST CASE

UNFACTORED REACTIONS	MAX MIN. COMPONENT REACTIONS
JT COMBINED SNOW	LIVE
H 593	117/0
F 593	356/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

CHORDS	FACTORED	VERT. LOAD	CSF (LBS)	CSF (LBS)	CSF (LBS)
FR-TO	0/32	-84.3	0.11 (1)	10.00	0.06 (3)
A-B	-464/0	-84.3	0.34 (1)	6.25	0.09 (1)
B-C	-464/0	-84.3	0.34 (1)	6.25	0.09 (1)
C-D	-464/0	-84.3	0.34 (1)	6.25	0.09 (1)
D-E	-464/0	-84.3	0.34 (1)	6.25	0.09 (1)
H-B	-683/0	0.0	0.07 (1)	7.81	0.09 (1)
F-D	-683/0	0.0	0.07 (1)	7.81	0.09 (1)
H-G	0/0	-28.0	0.25 (3)	10.00	0.09 (1)
G-F	0/0	-28.0	0.25 (3)	10.00	0.09 (1)

JOB NAME
280013

TRUSS NAME
T21

QUANTITY
1

PLY
2

JOB DESC.
43954 TRUSS DESC.

DRWG NO.

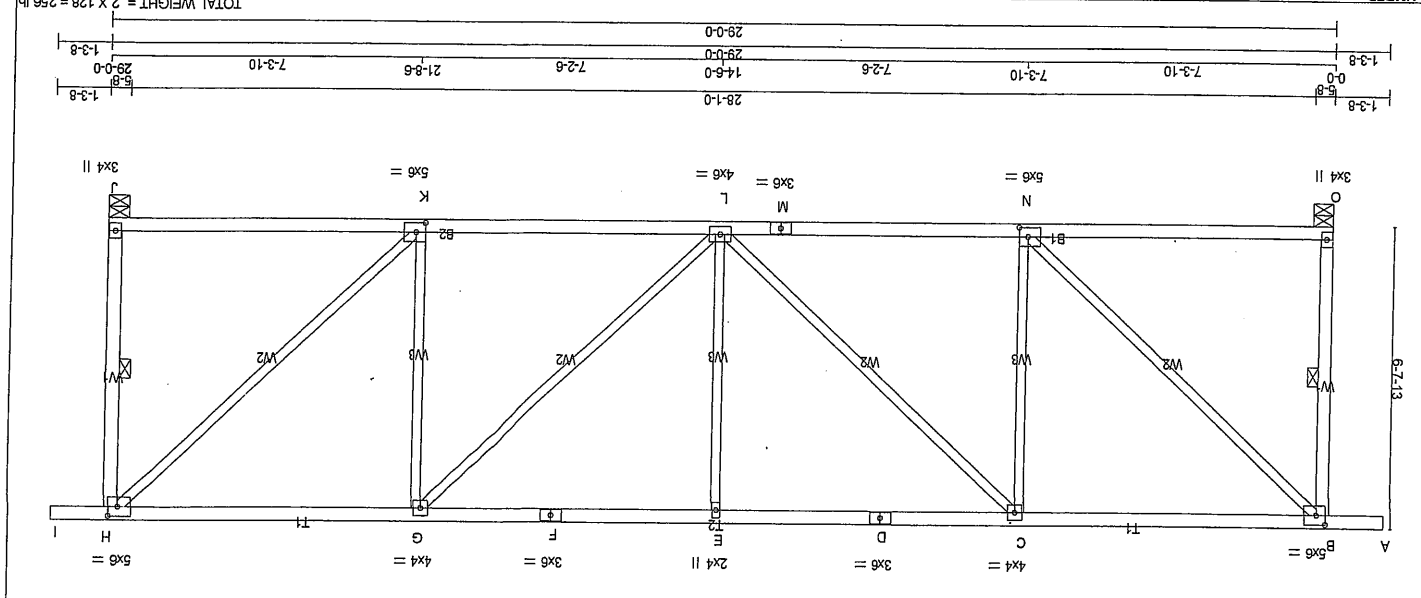
Tamarack Roof Truss, Burlington

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Version 8.030 S Oct 5 2016 MITTEK Industries, Inc. Wed May 10 00:33:42 2017 Page 2

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 101.4 lbs FACTORED DOWN AT
13-7-12, AND 101.4 lbs FACTORED DOWN AT
15-7-12, AND 101.4 lbs FACTORED DOWN AT
17-0-4 ON TOP CHORD, AND 1507.0 lbs
FACTORED DOWN AT 12-5-8, 71.7 lbs
FACTORED DOWN AT 13-7-12, 71.7 lbs
FACTORED DOWN AT 15-7-12, AND 71.7 lbs
FACTORED DOWN AT 17-0-4, AND 1406.0 lbs
FACTORED DOWN AT 17-8-9 ON BOTTOM
CHORD. DESIGN FOR UNSPECIFIED
CONNECTION(S) IS DELEGATED TO THE
BUILDING DESIGNER.



2nd
DWG NO. TAM21407-17
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 2 X 128 = 256 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
BOT CH. LL = 7.0 PSF
DL = 10.5 PSF
TOTAL LOAD = 43.1 PSF
SPACING = 24.0 IN. C/C

LOADING IN PLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH: PART 9 OF OBC 2012, BCBC 2012, ABC 2014 - CSA 086-09 - TPO 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFLECTION (LL) = L/360 (0.97) ALLOWABLE DEFLECTION (TL) = L/960 (0.57) CALCULATED DEFLECTION (TL) = L/999 (0.21)

CSP: TC=0.88 (C-E-1), BC=0.55 (L-N-2), WB=0.78 (G-K-1), SS=0.29 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LENS BEND=1.10 COMP=1.10 SHEAR=1.10 TENNS=1.10

COMPANION LIVE LOAD FACTOR = 0.50 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PL) MAX MIN MAX MIN

PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg

JSI METAL = 0.40 (M) (INPUT = 1.00)
JSI GRIP = 0.87 (K) (INPUT = 0.90)

DWG NO. 280013-17
STRUCTURAL COMPONENT ONLY

PROVINCE OF ONTARIO
S. KATSOULAKOS
LICENSED PROFESSIONAL ENGINEER

PLATES (Table in inches)

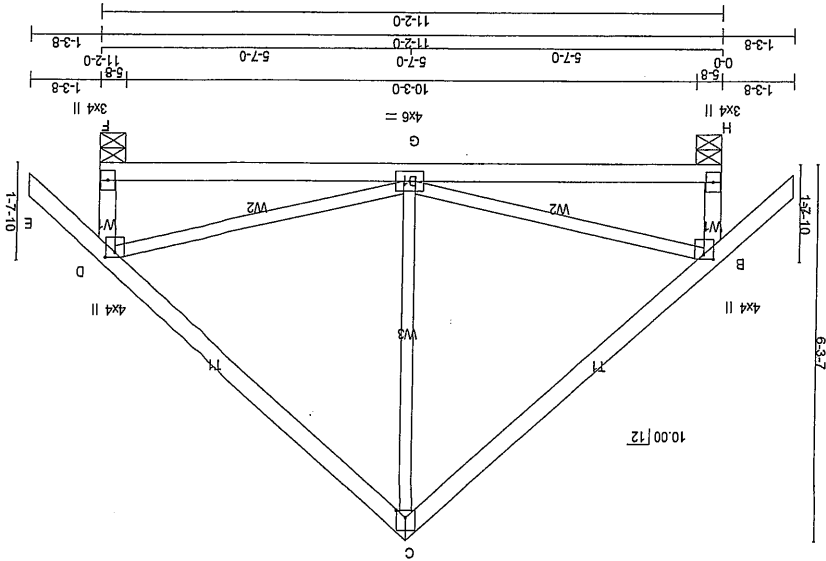
ALL WEBS 2x3 DRY SEASONED LUMBER.

DRY: SEASONED LUMBER.

PLATES (Table in inches)

DRY: SEASONED LUMBER.

Scale = 1:38.0



TOTAL WEIGHT = 6 X 50 = 298 lb

DESIGN CRITERIA

SPECIFIED LOADS: 25.6 PSF TOP CH. LL = 3.0 PSF BOT CH. LL = 7.0 PSF TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCS 2010

THIS DESIGN COMPLIES WITH: -PART 9 OF CBC 2012, BCBC 2012, ABC 2014

ALLOWABLE DEFLECTION (LL) = L/360 (0.37")

ALLOWABLE DEFLECTION (TL) = L/999 (0.03")

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

MAIL VALUES

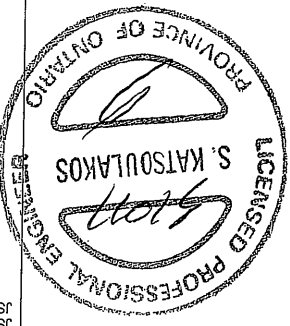
PLATE GRIP (DRY) SHEAR SECTION

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.85 (C) (INPUT = 0.90)

JSI METAL = 0.17 (B) (INPUT = 1.00)



BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FORCE

MEMB. FORCE

MEMB. FORCE

DRY, SEASONED LUMBER.

ALL WEBS 2x3 DRY

EXCEPT

CHORDS

N. L. G. A. RULES

PLATES (table in inches)

JT TYPE

PLATES

PLATES

PLATES

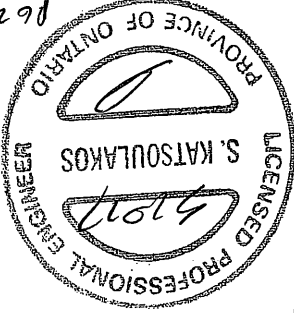
JOB NAME	280014	TRUSS NAME	T31	QUANTITY	1	PLY	2	JOB DESC.	43954	TRUSS DESC.		DRWG NO.	
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Tamarack Roof Truss, Burlington

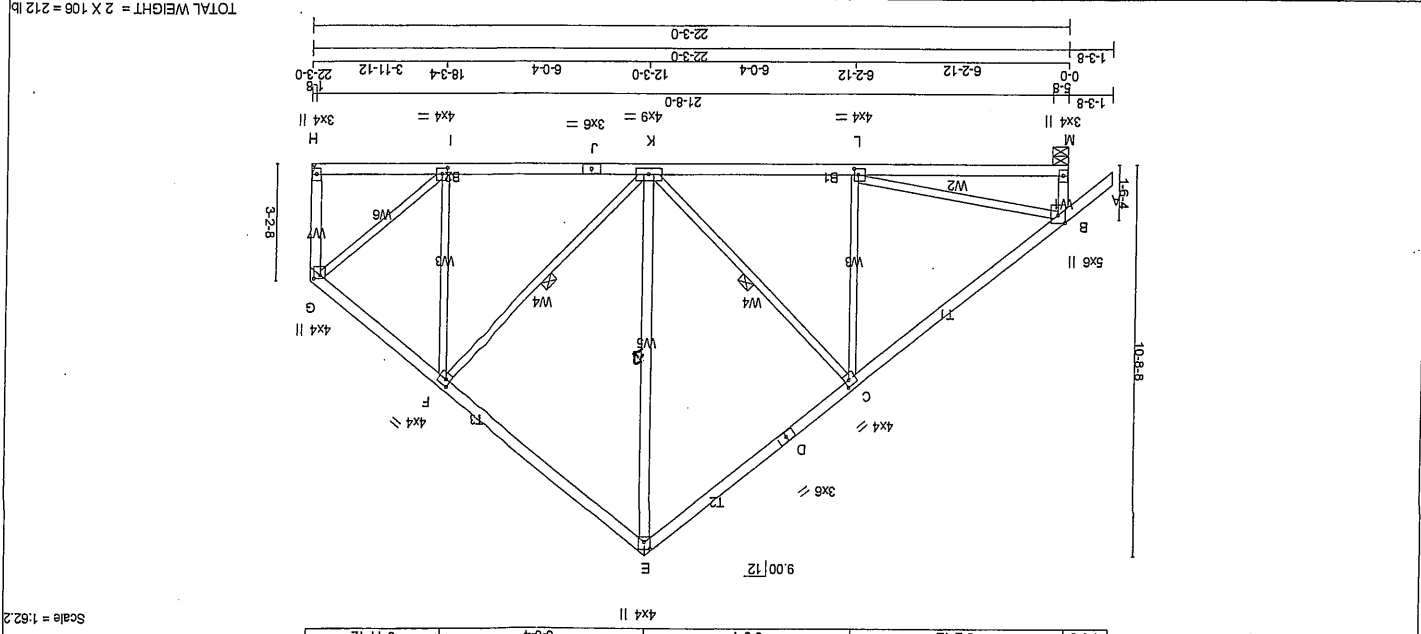
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Version 8.030 S Oct 5 2016 Mitek Industries, Inc. Wed May 10 00:34:26 2017 Page 2

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 105.2 lbs FACTORED DOWN AT 5-0-12
AND 106.8 lbs FACTORED DOWN AT 5-11-4 ON
TOP CHORD, AND 71.7 lbs FACTORED DOWN
AT 1-0-12, 71.7 lbs FACTORED DOWN AT
3-0-12, 71.7 lbs FACTORED DOWN AT 5-0-12,
AND 71.7 lbs FACTORED DOWN AT 5-11-4, AND
2179.5 lbs FACTORED DOWN AT 6-7-8 ON
BOTTOM CHORD. DESIGN FOR UNSPECIFIED
CONNECTION(S) IS DELEGATED TO THE
BUILDING DESIGNER.



22nd
DWG NO. TAM 21412
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 2 X 108 = 212 lb

DESIGN CRITERIA

SPACING = 240 IN. C/C	DESIGN CRITERIA
TOTAL LOAD = 46.1 PSF	
D.L. = 7.0 PSF	
BOT CH. LL = 10.5 PSF	
D.L. = 3.0 PSF	
TOP CH. LL = 26.6 PSF	
SPECIFIED LOADS:	
TOP CH. LL = 26.6 PSF	
D.L. = 3.0 PSF	
BOT CH. LL = 10.5 PSF	
D.L. = 7.0 PSF	
TOTAL LOAD = 46.1 PSF	

UNFACTORED REACTIONS

1ST CASE	MAX MIN. COMPONENT REACTIONS	SOIL
JT	COMBINED SNOW	DEAD
M	104	0/0
H	1025	234/0
JT	COMBINED SNOW	DEAD
M	104	0/0
H	1025	234/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

CHORDS

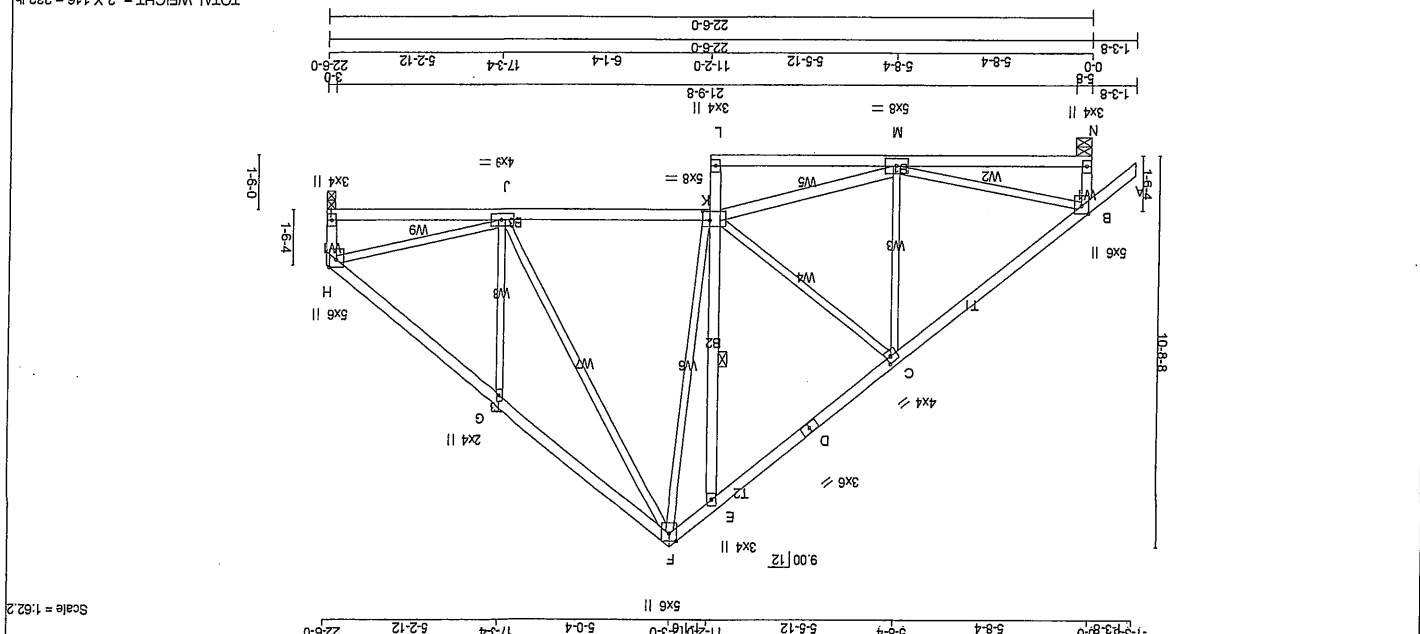
CHORDS	FACTORED	MEMB.	FORCE	VERT. LOAD	MAX	MEMB.	FORCE	MAX
FR-TO	0/35	(LBS)	CSF (LC)	UNBRAC	CSF (LC)	UNBRAC	CSF (LC)	UNBRAC
A-B	-84.3	0.11 (1)	10.00	-8.239	0.05 (5)	-513/0	0.24 (1)	0.10 (1)
B-C	-1260/0	-84.3	0.46 (1)	5.17	-513/0	0.24 (1)	0.10 (1)	0.10 (1)
C-D	-876/0	-84.3	0.43 (1)	5.97	0/610	0.10 (1)	0.10 (1)	0.10 (1)
D-E	-876/0	-84.3	0.43 (1)	5.97	-120/0	0.08 (1)	0.24 (1)	0.24 (1)
E-F	-866/0	-84.3	0.37 (1)	6.09	-393/2	0.24 (1)	0.24 (1)	0.24 (1)
F-G	-902/0	-84.3	0.31 (1)	6.09	0/1057	0.24 (1)	0.24 (1)	0.24 (1)
G-H	-1294/0	0.0	0.14 (1)	7.10	0/934	0.21 (1)	0.21 (1)	0.21 (1)
H-I	-1210/0	0.0	0.22 (1)	7.28				

PLATES (Table is in inches)

PLATES	JT TYPE	LEN	EDGE	W
B	TMVW+0	MT20	4.0	5.0
C	TMVW+0	MT20	4.0	6.0
D	TS-1	MT20	3.0	6.0
E	TTW+0	MT20	4.0	2.25
F	TMVW+0	MT20	4.0	1.00
G	TMVW+0	MT20	4.0	2.00
H	BMV+0	MT20	3.0	4.0
I	BMV+0	MT20	3.0	4.0
J	BS-1	MT20	3.0	9.0
K	BMVW+0	MT20	4.0	9.0
L	BMVW+0	MT20	4.0	2.00
M	BMV+0	MT20	3.0	4.0

TOUCHES EDGE OF CHORD

TOUCHES EDGE OF CHORD	FACTORED	MEMB.	FORCE	VERT. LOAD	MAX	MEMB.	FORCE	MAX
FR-TO	0/35	(LBS)	CSF (LC)	UNBRAC	CSF (LC)	UNBRAC	CSF (LC)	UNBRAC
A-B	-84.3	0.11 (1)	10.00	-8.239	0.05 (5)	-513/0	0.24 (1)	0.10 (1)
B-C	-1260/0	-84.3	0.46 (1)	5.17	-513/0	0.24 (1)	0.10 (1)	0.10 (1)
C-D	-876/0	-84.3	0.43 (1)	5.97	0/610	0.10 (1)	0.10 (1)	0.10 (1)
D-E	-876/0	-84.3	0.43 (1)	5.97	-120/0	0.08 (1)	0.24 (1)	0.24 (1)
E-F	-866/0	-84.3	0.37 (1)	6.09	-393/2	0.24 (1)	0.24 (1)	0.24 (1)
F-G	-902/0	-84.3	0.31 (1)	6.09	0/1057	0.24 (1)	0.24 (1)	0.24 (1)
G-H	-1294/0	0.0	0.14 (1)	7.10	0/934	0.21 (1)	0.21 (1)	0.21 (1)
H-I	-1210/0	0.0	0.22 (1)	7.28				



TOTAL WEIGHT = 2 X 116 = 232 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 3.0 PSF

LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014

- CSA 086-09

- T-PIG 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFLECTION = L/360 (0.75")

CALCULATED VERT. DEFLECTION = L/999 (0.08")

ALLOWABLE DEFLECTION = L/360 (0.75")

CALCULATED VERT. DEFLECTION = L/999 (0.13")

CSF: TC=0.36 (B-C:1), BC=0.36 (J-K:2), WB=0.25 (H-I:1), SSI=0.18 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 L.S. BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

MAX MIN MAX MIN

MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (K) (INPUT = 0.90)

JSI METAL = 0.26 (B) (INPUT = 1.00)

DRY: SEASONED LUMBER.

ALL WEBS EXCEPT

M - K 2x4 DRY No.2

SPF

1ST CASE UNFACTORED REACTIONS

MAX MIN COMPONENT REACTIONS

DEAD WIND PERM LIVE

0.0 0.0 0.0 0.0

233.0 225.0

0.0 0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.26 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FORCE (LBS)

FACTORED VERT. LOAD LCI MAX

CSF (LBS)

FROM TO

FR-TO

Edge - INDICATES REFERENCE CORNER OF PLATE

TOWARDS EDGE OF CHORD.

PLATES (Table in inches)

JT TYPE

W LEN Y X

LUS – Double Shear Joist Hangers



All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

MATERIAL: 18 gauge

FINISH: G90 galvanized

DESIGN:

- Factored resistances are in accordance with CSA 086-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given.
- The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

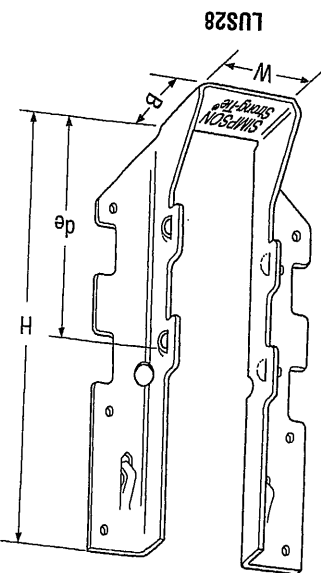
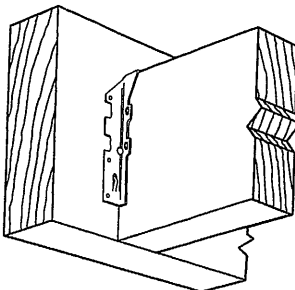
INSTALLATION:

- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

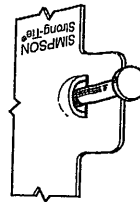
- These hangers cannot be modified.

Typical LUS Installation

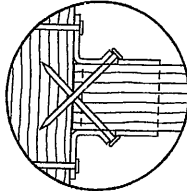


Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)	
		W	H	B	de	Face	Joist	Uplift	Normal
LUS24	18	1 1/8	3 3/4	1 1/4	1 5/8	4-10d	2-10d	710	1630
LUS24-2	18	3 3/4	3 3/4	2	1 13/16	4-16d	2-16d	835	2020
LUS26	18	1 1/8	3 3/4	1 1/4	1 5/8	4-10d	4-10d	1420	2170
LUS26-2	18	3 3/4	4 7/8	2	4	4-16d	4-16d	1720	2595
LUS26-3	18	4 3/4	4 3/4	2	3 3/4	4-16d	4-16d	1720	2595
LUS28	18	1 1/8	6 5/8	1 1/4	3 3/4	6-10d	4-10d	1420	2520
LUS28-2	18	3 3/4	7	2	4	6-16d	4-16d	1720	3325
LUS28-3	18	4 3/4	6 1/4	2	3 1/4	6-16d	4-16d	1720	3325
LUS210	18	1 1/8	7 3/4	1 1/4	3 3/4	8-10d	4-10d	1420	2785
LUS210-2	18	3 3/4	9	2	6	8-16d	6-16d	2580	4500
LUS210-3	18	4 3/4	8 3/4	2	5 1/4	8-16d	6-16d	2580	4500
LUS210-3	18	4 3/4	8 3/4	2	5 1/4	8-16d	6-16d	2580	4500

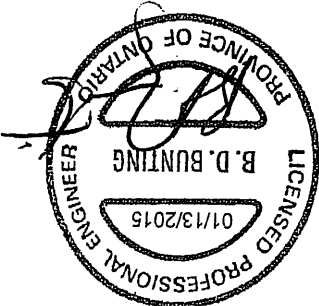
1. de is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models). U.S. Patent 5,603,580



Double Shear Nailing Top View



800-999-5099
www.strongtie.com

This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2015. This information is updated periodically and should not be relied upon after December 31, 2016. contact Simpson Strong-Tie for current information and limited warranty or see www.strongtie.com.
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T-SPEC LUS15 1/15 exp. 12/16

LIMIT STATES DESIGN



HUS/LJS – Double Shear Joist Hangers

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: See table
FINISH: G90 galvanized

DESIGN:

- Factored resistances are in accordance with CSA 086-14
- Uplift resistances have been increased 15% No further increase is permitted
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

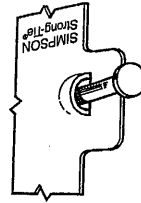
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/4" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

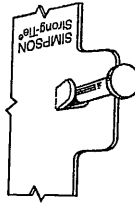
- See current catalogue for options

Model No.	Ga	Dimensions (in)					Fasteners		Factored Resistance (lbs)	
		W	H	B	d _g	d _g	Uplift	Normal	D. Fir-L	S-P-F
LJS26DS	18	1 1/4	5	3 1/2	4%	6-16d	(K _g =1.15)	(K _g =1.00)	4265	1460
HUS26	16	1 1/4	5%	3	3 5/16	6-16d	(K _g =1.15)	(K _g =1.00)	2705	2065
HUS28	16	1 1/4	7 3/32	3	6 3/32	8-16d	(K _g =1.15)	(K _g =1.00)	3605	5365
HUS210	16	1 1/4	9 3/32	3	7 3/32	10-16d	(K _g =1.15)	(K _g =1.00)	4505	5795
HUS1.8/1.10	16	1 1/4	1%	3	8	10-16d	(K _g =1.15)	(K _g =1.00)	4505	6450

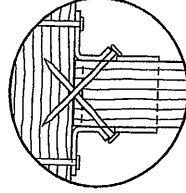
1. d_g is the distance from the seat of the hanger to the highest joist nail.



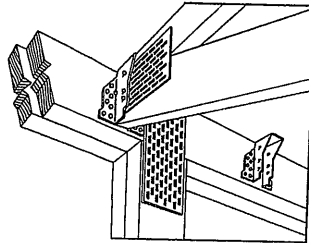
Dome Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,580



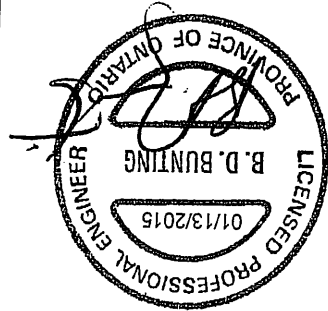
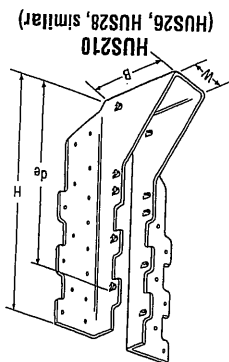
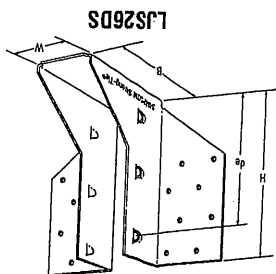
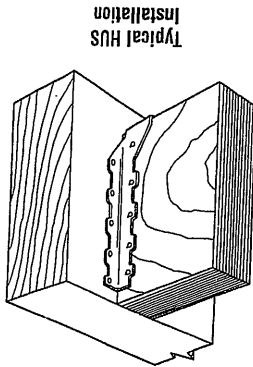
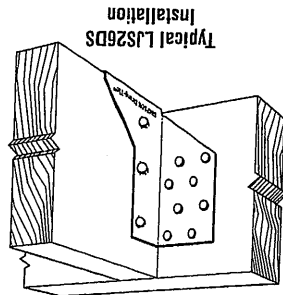
Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



Typical HUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)



800-999-5099
www.strongtie.com

This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2016. This information is updated periodically and should not be relied upon after December 31, 2016. contact Simpson Strong-Tie for current information and limited warranty or see www.strongtie.com.
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T-SPEC-HUS15 1/15 exp. 12/16

LIMIT
STATES
DESIGN



SIMPSON
Strong-Tie

HGUS – Double Shear Joist Hangers



All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: 12 gauge
FINISH: G90 galvanized

DESIGN:

- Factored resistances are in accordance with CSA 086-14
- Uplift resistances have been increased 15%.
- No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

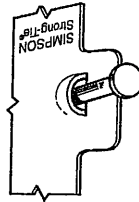
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

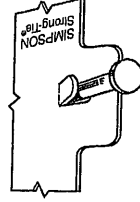
- See current catalogue for options

Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)	
		W	H	B	d ₁	Face	Joist	Uplift	Normal
HGUS26	12	1 1/2	5 3/4	5	4 3/4	20-16d	8-16d	2685	6625
HGUS26-2	12	3 3/8	5 7/8	4	4	20-16d	8-16d	4385	8950
HGUS26-3	12	4 1/8	5 1/2	4	4	20-16d	8-16d	4385	8950
HGUS26-4	12	6 3/8	5 1/2	4	4	20-16d	8-16d	4385	8950
HGUS28	12	1 1/2	7 1/8	5	6	36-16d	12-16d	3310	7675
HGUS28-2	12	3 3/8	7 3/8	4	6 3/8	36-16d	12-16d	6070	12980
HGUS28-3	12	4 1/8	7 3/8	4	6 3/8	36-16d	12-16d	6070	12980
HGUS28-4	12	6 3/8	7 3/8	4	6 3/8	36-16d	12-16d	6070	12980
HGUS210-2	12	3 3/8	9 3/8	4	8 3/8	46-16d	16-16d	6840	14645
HGUS210-3	12	4 1/8	9 3/8	4	8 3/8	46-16d	16-16d	6840	14645
HGUS210-4	12	6 3/8	9 3/8	4	8 3/8	46-16d	16-16d	6840	14645
HGUS212-4	12	6 3/8	10 3/8	4	10 3/8	56-16d	20-16d	7640	14995
HGUS214-4	12	6 3/8	12 3/8	4	11 3/8	66-16d	22-16d	10130	16400

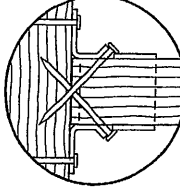
1. d_g is the distance from the seat of the hanger to the highest joist nail.



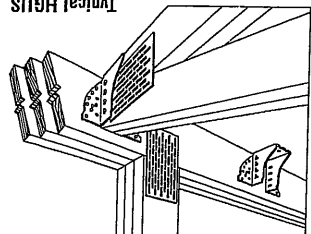
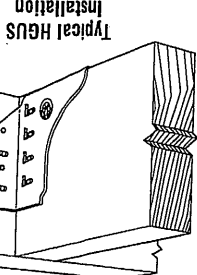
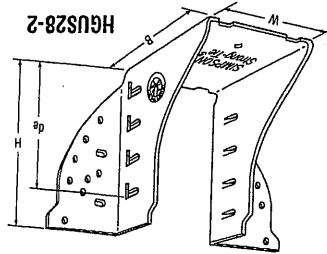
Dome Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,580



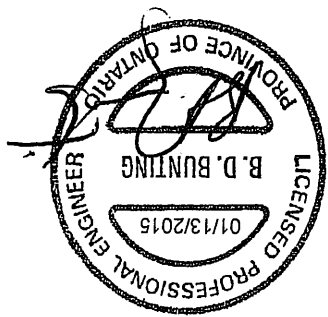
Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



Typical HGUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)



800-999-5099
www.strongtie.com

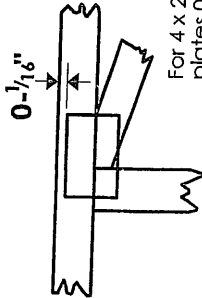
This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2015. This information is updated periodically and should not be relied upon after December 31, 2016. contact Simpson Strong-Tie for current information and limited warranty or see www.strongtie.com.
T-SPECHGUS15 1/15 exp. 12/16
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Symbols

PLATE LOCATION AND ORIENTATION

Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/8\" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

* Plate location details available in Mitek software or upon request.

PLATE SIZE

4 X 4

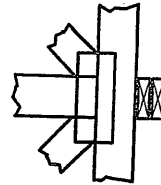
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



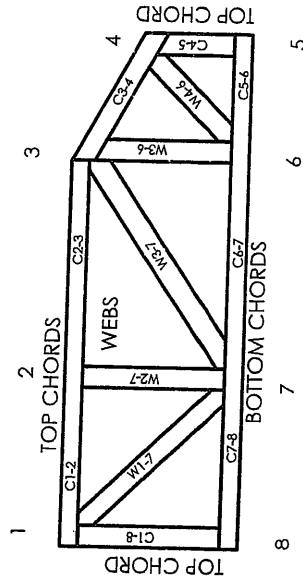
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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Mitek
POWER TO PERFORM™

Mitek Engineering Reference Sheet: MIL-7473C rev. 10-08

General Safety Notes

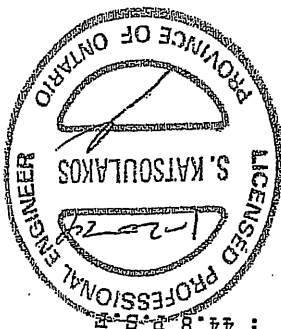
Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

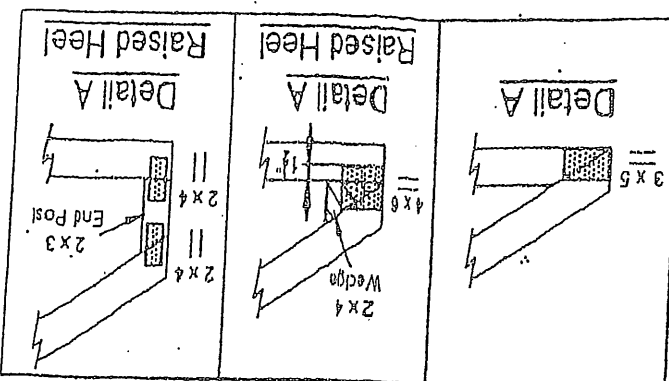
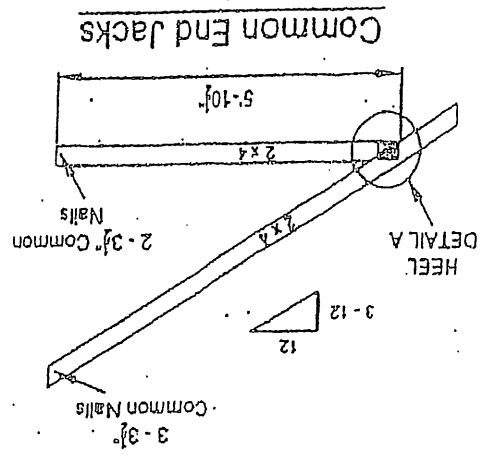
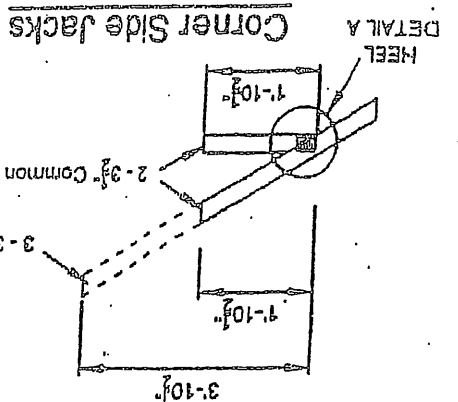
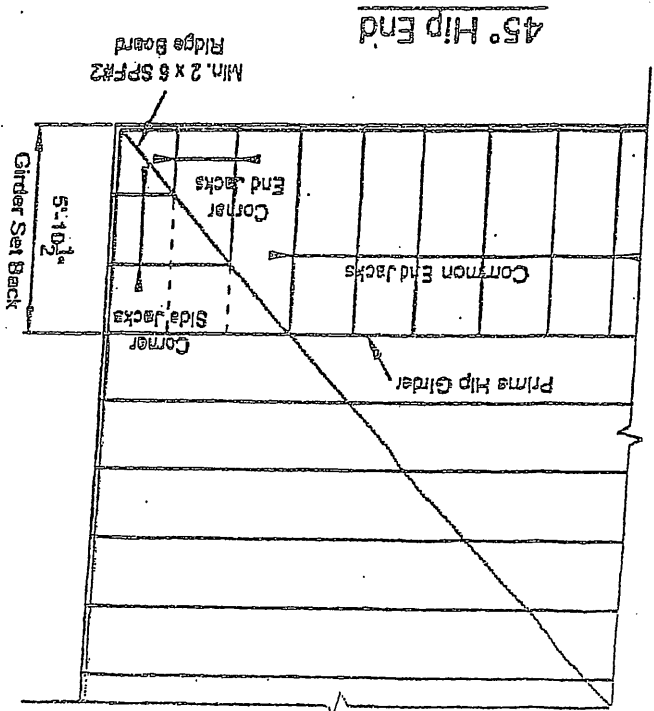
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

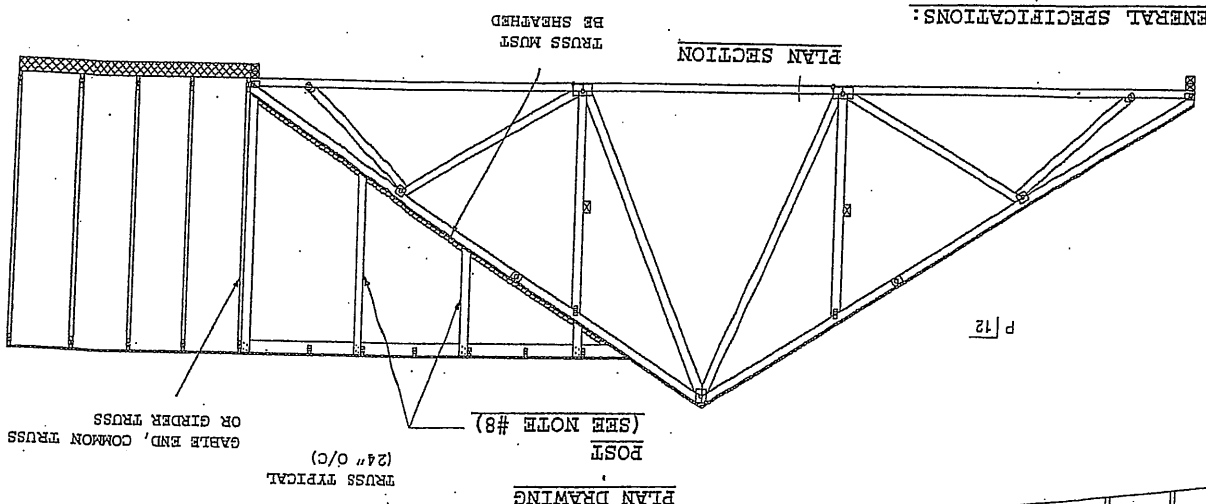
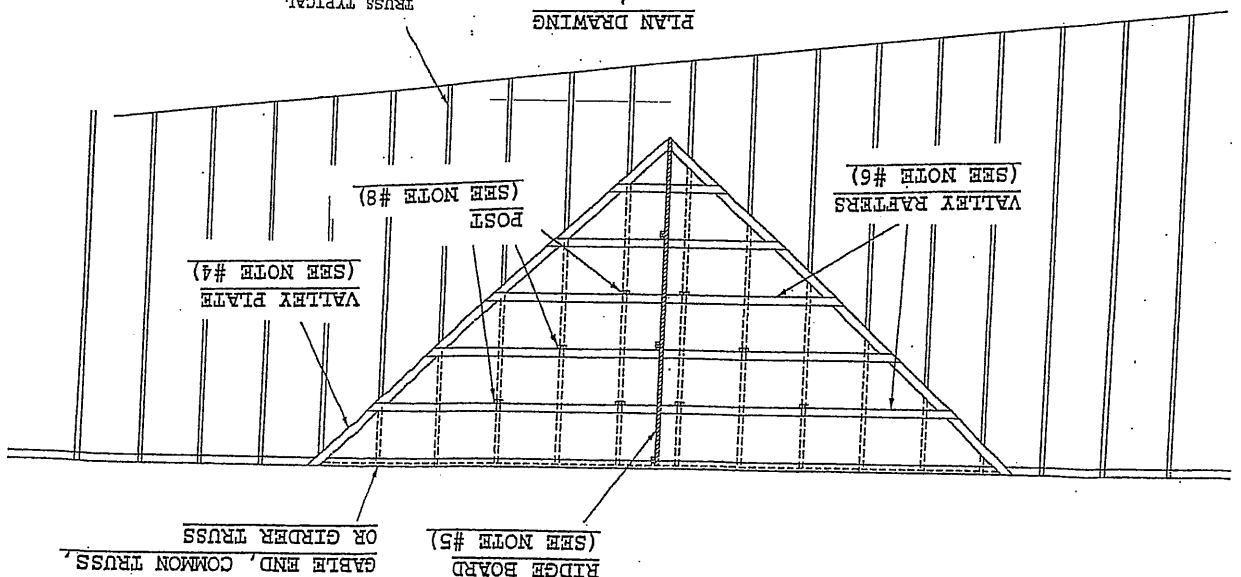
DESIGN LOAD:	
TOP CHORD LIVE LOAD	: 34.8 P.S.F.
TOP CHORD DEAD LOAD	: 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	: 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	: 7.0 P.S.F.
TOTAL LOAD	: 44.8 P.S.F.



DESIGN NO. TAM 3495.14
 STRUCTURAL
 COMPONENT ONLY



CONVENTIONAL VALLEY FRAMING DETAIL



GENERAL SPECIFICATIONS:

(1) WITH THE BASE TRUSSES ERECTED (INSTALLED), APPLY SHEATHING

(2) BRACE BOTTOM CHORD AND WEB MEMBERS AS PER PRE-ENGINEERED

TRUSS DESIGNS.

(3) DEFINE VALLEY RIDGE BY RUNNING A LEVEL STRING FROM THE

INTERSECTING RIDGE OF THE (a) GABLE END, (b) GIRDER TRUSS OR

(c) COMMON TRUSS TO THE ROOF SHEATHING.

(4) INSTALL 2 X 6 VALLEY PLATES ON FLAT, FASTEN TO EACH SUPPORTING

TRUSS WITH (2) 16d (3.5" X 0.131") NAILS.

(5) SET 2 X 6 #2 RIDGE BOARD (MAX. 10'-0" RIDGE) OR 2 X 8 #2

RIDGE BOARD (MAX. 20'-0" RIDGE), SUPPORT RIDGE BOARD WITH 2 X 4

POSTS SPACED 48" O/C. BEVEL BOTTOM OF POST TO SET EVENLY ON THE

SHEATHING. FASTEN POST TO RIDGE WITH (4) 10d (3" X 0.131") NAILS.

(6) FASTEN POST TO ROOF SHEATHING WITH (3) 10d (3" X 0.131") NAILS.

(7) FRAME VALLEY RAFTERS FROM VALLEY PLATE TO RIDGE BOARD. MAXIMUM

RAFTER SPACING IS 24" O/C. FASTEN VALLEY RAFTER TO RIDGE BEAM WITH

(3) 16d (3.5" X 0.131") NAILS. FASTEN VALLEY RAFTER TO VALLEY

PLATE WITH (3) 16d (3.5" X 0.131") JOE-NAILS.

(8) SUPPORT THE VALLEY RAFTERS WITH 2 X 4 POSTS AT 48" O/C (OR LESS)

ALONG EACH RAFTER. INSTALL POSTS IN A STAGGERED PATTERN AS SHOWN

ON PLAN DRAWING. ALIGN POSTS WITH TRUSSES BELOW. FASTEN VALLEY

RAFTER TO POST WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST

(9) POSTS SHALL BE 2 X 4 #2 SFE OR BETTER. POSTS EXCEEDING 75' IN HEIGHT

SHALL BE INCREASED TO 4 X 4 #2 SFE, OR BETTER, OR BE PRE-ASSEMBLED

TWO (2) PLY 2 X 4 #2 SFE OR BETTER FASTENED TOGETHER WITH 2 ROWS OF

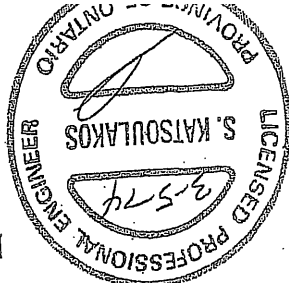
10d (3" X 0.131") NAILS AT 6" O/C.

(10) MAINTAIN A MINIMUM 3/4" LUMBER EDGE DISTANCE WHEN NAILING. NAIL SPACING

SHALL APPROXIMATE A MINIMUM 1-3/4" O/C OR MORE UNLESS NOTED OTHERWISE.

ALL CONSTRUCTION TO CONFORM TO ONTARIO BUILDING CODE (CURRENT ADDITION)

AT ALL TIMES.



THIS NO. 6305.14
STRUCTURAL
COMPONENT ONLY

- NOTES:
- (10) 48" O/C (MAXIMUM POST SPACING.
 - (11) ROOF LIVE LOAD = 34.8 PSF (MAX.)
 - (12) ROOF DEAD LOAD = 10.0 PSF (MAX.)
 - (13) PART 9 APPLICATION ONLY
 - (14) PART 4 APPLICATION ONLY
 - (15) PROFESSIONAL ENGINEER.
 - (16) BASE TRUSS SPACING (24" O/C MAX.)
 - (17) ALL PRE-ENGINEERED BASE TRUSS
 - (18) COMPONENTS TO BE SEALED BY LICENSED
 - (19) PROFESSIONAL ENGINEER AND THIS DETAIL
 - (20) TO BE VERIFIED AND APPROVED BY SAME
 - (21) WHEN RIDGE BOARD LENGTH EXCEEDS 12'-0".
 - (22) - MINIMUM.
 - (23) - MINIMUM.
 - (24) ALL VALLEY RAFTERS: P = 4 (4/12) - MINIMUM.

Micro City Engineering Services Inc.
(BCIN: 26064; FIRM BCIN: 29991)
RR #1, Po Box 61
Glencoe, Ontario
N0L 1M0
(519) 287 - 2242; Fax: (519) 287 - 5750 (Call)



Responsibilities:

Micro City Engineering Services is responsible for the design of trusses as individual components. It is the responsibilities of others to ascertain that the design loads utilized on this (these) drawing(s) meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdiction over such decisions. All dimensions are to be verified by the owner, contractor, architect, or other authority having input over such decisions prior to truss component manufacture. At no time shall Micro City Engineering Services Inc. or its employees be responsible for dimension errors.

Micro City Engineering Services Inc. bears no responsibility for the erection of any truss components. Persons erecting truss components are cautioned to seek professional advice regarding temporary and permanent bracing systems and to be totally familiar with all aspects of truss erection prior to proceeding on any truss component erection job. Any bracing shown on Micro City Engineering Services Inc. or Tamarack Roof Trusses Inc. sealed or unsealed truss component drawings is specified for the single truss component in question and is identified as an integral part of the design for that particular truss component but is not meant to represent the only required bracing for that particular truss component when installed as a component in a series of truss components in a roof truss system.

It is the truss manufacturer's responsibility to ensure that trusses are manufactured in accordance with Micro City Engineering Services Inc. specifications outlined below:

SPECIFICATIONS:

Truss components sealed by Micro City Engineering Services Inc. must conform to the relevant sections of the current Building Code of Ontario and Canada (Part 4 or Part 9) or the current Farm Building Code of Canada in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the Truss Plate Institute of Canada (TPIC). All unit lumber and nailing stresses identified on truss component design drawings and/or used in the design of individual truss components shall conform to the current CSA Wood Design standard identified in the current Building Code and TPIC Design Standards.

The lumber used to manufacture any truss component is to conform to the specified size and grade identified on the truss drawing on the truss drawing.

The lumber used in the manufacture of any truss component is not to exceed 19% during its service use unless specifically noted specifically noted in the manufacture of any truss component is not to be treated with any chemicals during its service life unless specifically noted on the truss drawing.

Connector plates shall be applied to both faces of the truss component at each joint and shall be positioned exactly as specified. The top chord of any truss component is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the sealed truss component drawing but not exceeding 24" o/c (Part 9 design) and not exceeding 48" o/c (Part 4 or Agricultural design).

When a truss component is to be installed with no rigid ceiling attached directly to the bottom chord, then the bottom chord is to be laterally braced at intervals not exceeding 3m (or 10'-0").

All sealed or unsealed truss component drawings provided by Micro City Engineering Services Inc. Or Tamarack Roof Trusses Inc. should be read in conjunction with the following:

Warning-Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTBK REFERENCE PAGE MII-7473C rev 10-08 BEFORE USE. Design valid for use only with Mittek connectors. This design is based only upon parameters shown, and is for individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpinc.ca and BCSI Building Component Safety Information available from the Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA, 22314.