ASPHALT SHINGLES
12"FINISH O.H.
R.T.M.C
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:

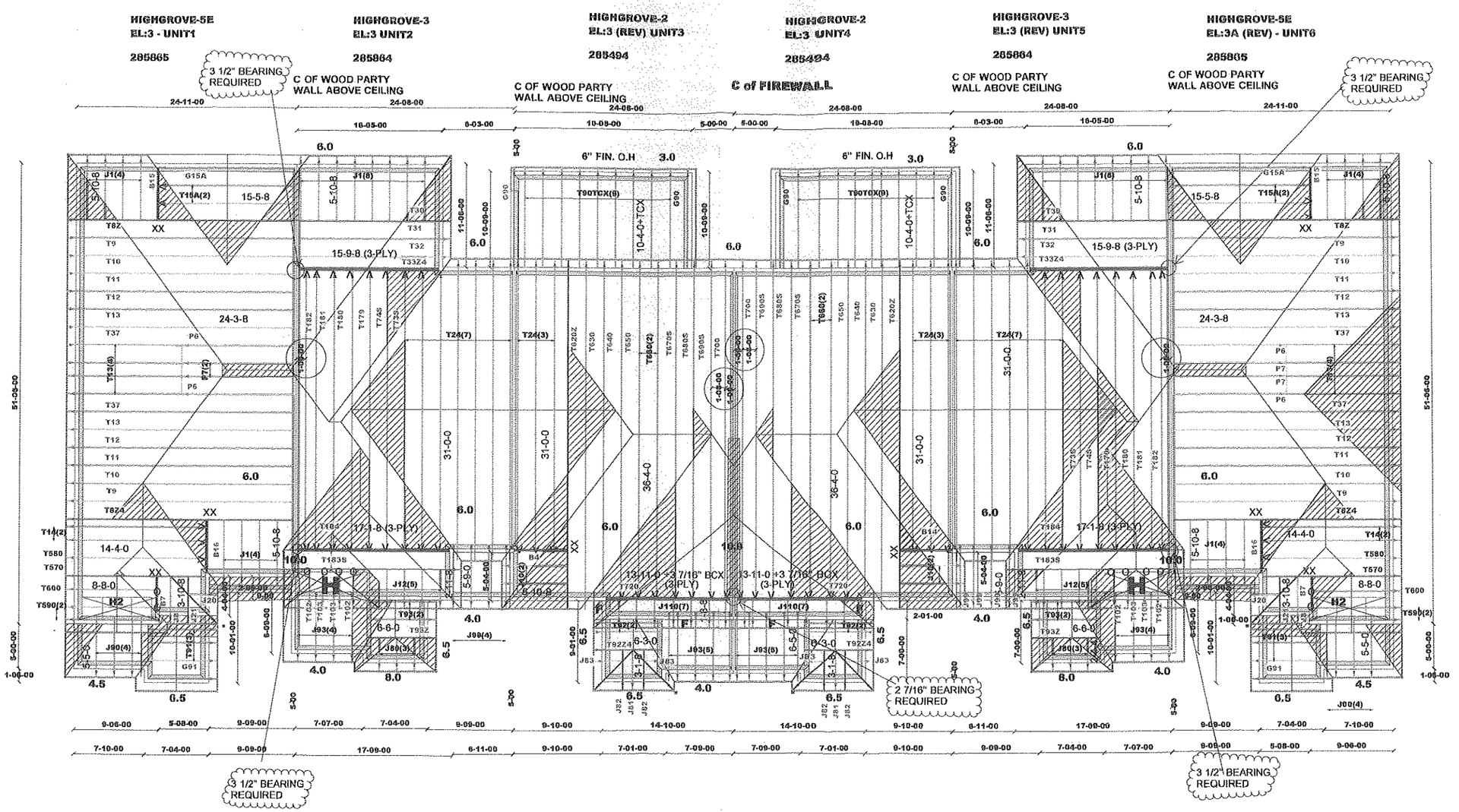
HGUS26-2 -(XX)
LJS26DS -(V)
LUS24 -(O)

9/12 PITCHES (TYP.)
UNLESS NOTED

DENOTES:
H -2'-0-0 HIGHER PLATE
H2 -1'-8"-0 RAISED CEILING
F -12" HIGHER FASCIA ONLY
ALL B- 2-2X10 BEAMS (FLUSH)

T-170534

DENOTES:
CONVENTIONAL
FRAMING



ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION
ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"o.c.
WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.
POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE
BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN LOADS:

SNOW LOAD 25.6 PSF
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

TAMARACK	Job Track: 43954	Builder / Location: GREENPARK HOMES/ WATERDOWN	Model / Elevation: BLOCK211 / UNIT1TO6 (REVISED)
	Layout ID: 285876	Project: RUSSEL GARDENS PHASE 2	Mitek ver 7.4.2 C:\MITEK\CA742AU OBS\B13000000006
	Plan Log: 93446	Date: 4/18/2018 Designer: sonny	

TAMARACK LUMBER INC
ALPA LUMBER GROUP

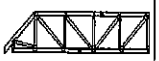
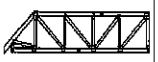
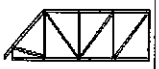
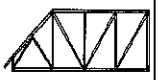
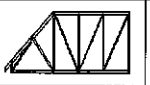
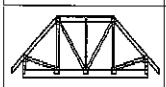
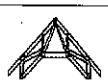
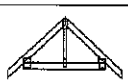
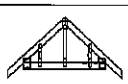
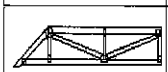
Delivery Shiplist

DATE	04/18/18
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 285865 LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH 2 SUB-BUILDER:
MODEL: HIGHGROVE 5E ELEVATION: 3(REV)-UNIT6 OR 3-UNIT1

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T8Z HALF HIP	8.00 0.00	24-03-08	04-01-04	2 X 6	2 X 6	01-03-08 00-00-00	01-04-13 04-01-04	143.26 88.34		
	1	T8Z4 HALF HIP	8.00 0.00	24-03-08	04-01-04	2 X 6	2 X 6	01-03-08 00-00-00	01-04-13 04-01-04	143.26 88.34		
	2	T9 HALF HIP	8.00 0.00	24-03-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-01-04	197.10 123.66		
	2	T10 HALF HIP	8.00 0.00	24-03-08	06-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 06-01-04	212.66 133.34		
	2	T11 HALF HIP	8.00 0.00	24-03-08	07-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 07-01-04	225.56 142.68		
	2	T12 HALF HIP	8.00 0.00	24-03-08	08-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 08-01-04	234.64 146.34		
	6	T13 HALF HIP	8.00 0.00	24-03-08	09-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 09-01-04	737.28 454.98		
	2	T37 HALF HIP	8.00 0.00	24-03-08	10-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 10-01-04	257.54 159.34		
	2	T15A COMMON	8.00 0.00	15-05-08	06-07-08	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	129.00 82.66		
	1	G15A COMMON	8.00 0.00	15-05-08	06-07-08	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	67.52 44.33		
	2	T14 COMMON	8.00 0.00	14-04-00	06-02-02	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	123.52 78.00		
	1	T570 HIP GIRDER	8.00 0.00	14-04-00	03-11-13	2 X 4	2 X 6	01-03-08 01-03-08	01-04-13 01-04-13	70.63 46.34		
	1	T580 HIP	8.00 0.00	14-04-00	05-03-13	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	62.73 39.33		
	2	T590 ROOF	8.00 8.00	08-08-00	04-03-08	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	83.72 58.68		
	1	T600 COMMON	8.00 0.00	08-08-00	04-03-08	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	37.48 23.67		
	3	T91 COMMON	6.50 0.00	06-06-00	02-04-09	2 X 4	2 X 4	01-03-08 01-03-08	00-07-07 00-07-07	66.60 45.00		
	1	G91 COMMON	6.50 0.00	06-06-00	02-04-09	2 X 4	2 X 4	01-03-08 01-03-08	00-07-07 00-07-07	23.39 16.00		
	2	P6 PIGGYBACK	8.00 0.00	11-11-15	01-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 01-10-08	80.22 51.34		

TAMARACK LUMBER INC
ALPA LUMBER GROUP

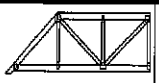
Delivery Shiplist

DATE	04/18/18
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 285865 LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH2 SUB-BUILDER:
MODEL: HIGHGROVE 5E ELEVATION: 3(REV)-UNIT6 OR 3-UNIT1

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	2	P7 PIGGYBACK	8.00 0.00	11-11-15	02-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 02-10-08	85.52 56.00		
	8	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	134.32 85.36		
	1	J8 JACK-OPEN	8.00 0.00	03-10-08	03-11-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 03-11-13	12.81 8.00		
	1	J20 JACK-OPEN	8.00 0.00	01-10-08	02-07-02	2 X 4	2 X 4	01-03-08 -00-01-01	01-04-13 00-03-08	9.17 5.67		
	1	J21 JACK-OPEN	8.00 0.00	03-10-08	02-07-02	2 X 4	2 X 4	01-03-08 -02-01-01	01-04-13 00-03-08	12.97 8.00		
	4	J90 JACK	4.50 0.00	05-05-00	02-05-00	2 X 4	2 X 4	01-03-08 00-00-00	00-04-10 02-05-00	57.88 37.32		

TOTAL # TRUSS= **51.00** TOTAL BFT OF ALL TRUSSES= **2022.72** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3208.78** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	HGUS26-2	
5	Hangers	LJS26DS	
2	Hangers	LUS24	

TOTAL # ITEMS= **10.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

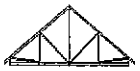
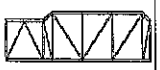
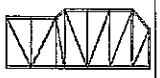
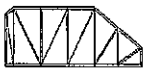
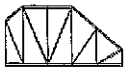
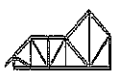
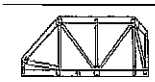
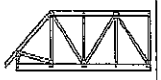
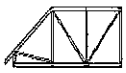
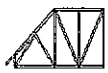
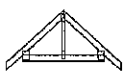
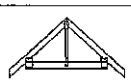
Delivery Shiplist

DATE	09/07/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 285864 LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH 2 SUB-BUILDER:
MODEL: HIGHGROVE 3 ELEVATION: 3-UNIT2 OR 3(REV) UNIT5

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	7	T24 COMMON	6.00 / 0.00	31-00-00	08-11-00	2 X 4	2 X 4	01-03-08 / 01-03-08	01-02-00 / 01-02-00	850.85 / 535.50		
	1	T73S ROOF	6.00 / 0.00	31-00-00	06-01-02	2 X 4	2 X 4	00-00-00 / 00-00-00	05-02-08 / 04-11-08	141.76 / 90.33		
	1	T74S ROOF	6.00 / 0.00	31-00-00	07-05-02	2 X 4	2 X 4	00-00-00 / 00-00-00	06-06-08 / 04-11-08	158.68 / 99.33		
	1	T179 HIP	6.00 / 0.00	31-00-00	07-10-08	2 X 4	2 X 4	00-00-00 / 00-00-00	06-11-00 / 02-07-00	148.46 / 94.17		
	1	T180 HIP	6.00 / 0.00	31-00-00	09-02-08	2 X 4	2 X 4	00-00-00 / 00-00-00	06-11-00 / 02-07-00	151.53 / 94.50		
	1	T181 HIP	6.00 / 0.00	31-00-00	10-06-08	2 X 4	2 X 4	00-00-00 / 00-00-00	06-11-00 / 02-07-00	158.15 / 98.17		
	1	T182 HIP	6.00 / 0.00	31-00-00	11-03-13	2 X 4	2 X 4	00-00-00 / 00-00-00	06-11-00 / 02-07-00	162.00 / 100.67		
	1	T183S ROOF	8.00 / 0.00	17-01-08	06-00-13	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 03-06-08	76.28 / 49.33		
	1 3 Ply	T184 HIP GIRDER	8.00 / 0.00	17-01-08	04-08-08	2 X 6	2 X 6	00-00-00 / 00-00-00	01-04-13 / 03-06-08	323.25 / 202.02		
	1	T30 HALF HIP	8.00 / 0.00	15-09-08	04-01-04	2 X 4	2 X 6	01-03-08 / 00-00-00	01-04-13 / 04-01-04	77.31 / 47.50		
	1	T31 HALF HIP	8.00 / 0.00	15-09-08	05-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 05-01-04	67.94 / 43.00		
	1	T32 HALF HIP	8.00 / 0.00	15-09-08	06-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 06-01-04	74.63 / 46.67		
	1 3 Ply	T33Z4 HALF HIP	8.00 / 0.00	15-09-08	06-08-12	2 X 6	2 X 6	00-00-00 / 00-00-00	01-04-13 / 06-08-12	341.19 / 201.00		
	2	T102 HALF HIP	10.00 / 0.00	02-11-08	02-01-08	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 04-01-08	35.80 / 25.00		
	2	T103 HALF HIP	10.00 / 0.00	02-11-08	03-05-08	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 05-05-08	42.06 / 28.66		
	2	T93 COMMON	6.50 / 0.00	06-06-00	02-04-09	2 X 4	2 X 4	01-03-08 / 01-03-08	00-07-07 / 00-07-07	44.40 / 30.00		
	1	T93Z COMMON	6.50 / 0.00	06-06-00	02-04-09	2 X 4	2 X 4	01-03-08 / 01-03-08	00-07-07 / 00-07-07	22.20 / 15.00		
	8	J1 JACK-OPEN	6.00 / 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 04-01-04	134.32 / 85.36		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

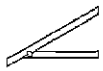
Delivery Shiplist

DATE	09/07/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 285864 LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH2 SUB-BUILDER:
MODEL: HIGHGROVE 3 ELEVATION: 3-UNIT2 OR 3(REV) UNIT 5

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	5	J12 JACK-OPEN	8.00 / 0.00	02-11-08	03-04-08	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 00-05-03	52.55 / 33.35		
	3	J80 JACK-OPEN	8.00 / 0.00	02-04-04	02-04-07	2 X 4	2 X 4	01-03-08 / 00-00-00	00-09-10 / 02-04-07	24.93 / 18.00		
	4	J93 MONOPITCH	4.00 / 0.00	06-05-00	02-05-10	2 X 4	2 X 4	01-03-08 / 00-00-00	00-03-15 / 02-05-10	72.64 / 46.68		
	4	J99 MONOPITCH	4.00 / 0.00	05-09-00	02-02-15	2 X 4	2 X 4	01-03-08 / 00-00-00	00-03-15 / 02-02-15	60.52 / 37.32		

TOTAL # TRUSS= 54 TOTAL BFT OF ALL TRUSSES= 2021.56 BFT. TOTAL WEIGHT OF ALL TRUSSES= 3221.45 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
4	Hangers	LUS24	
17	Hangers	LJS26DS	

TOTAL # ITEMS= 21

TAMARACK JOISTS INC.

Delivery Shiplist

DATE	07/03/18
SALES REP	Mario

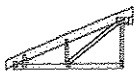
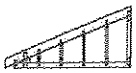
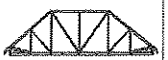
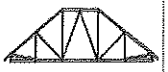
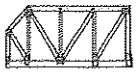
JOB TRACK: 43954 LAYOUT ID: 285494 LOCATION:

BUILDER: GREENPARK - RUSSELL GARDENS PH SUB-BUILDER:

MODEL: HIGHGROVE2 ELEVATION: 3-UNIT4 OR 3-UNIT 3,4

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	3	T24 COMMON	6.00 0.00	31-00-00	08-11-00	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	364.65 229.50		
	9	T90TCX MONOPITCH	3.00 0.00	10-04-00	03-05-09	2 X 6	2 X 4	00-09-08 00-00-00	00-08-04 03-05-09	404.91 272.97		
	2	G90 MONOPITCH	3.00 0.00	10-04-00	03-03-04	2 X 6	2 X 4	00-09-08 00-00-00	00-08-04 03-03-04	81.80 53.34		
	2	T92 COMMON	6.50 0.00	06-03-00	02-03-12	2 X 4	2 X 4	01-03-08 01-03-08	00-07-07 00-07-07	43.08 30.00		
	1	T92Z4 COMMON	6.50 0.00	06-03-00	02-03-12	2 X 4	2 X 4	01-03-08 01-03-08	00-07-07 00-07-07	21.54 15.00		
	1	T620Z HIP GIRDER	6.00 0.00	36-04-00	05-03-13	2 X 4	2 X 6	01-03-08 01-03-08	01-02-00 01-02-00	213.66 131.66		
	1	T630 HIP	6.00 0.00	36-04-00	06-07-13	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	146.04 92.83		
	1	T640 HIP	6.00 0.00	36-04-00	07-11-13	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	148.64 93.00		
	1	T650 HIP	6.00 0.00	36-04-00	09-03-13	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	156.18 98.00		
	2	T660 COMMON	6.00 0.00	36-04-00	10-03-00	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	293.96 180.00		
	1	T670S ROOF	6.00 0.00	36-04-00	10-03-00	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-04-08	161.65 100.67		
	1	T680S ROOF	6.00 0.00	36-04-00	10-03-00	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-04-08	173.83 108.67		
	1	T690S ROOF	6.00 0.00	36-04-00	10-01-13	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-04-08	178.48 109.83		
	1	T700 ROOF	6.00 0.00	36-04-00	11-03-02	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-04-08	188.58 116.33		
	1 3 Ply	T720 HALF HIP	8.00 0.00	13-11-00	04-00-12	2 X 4	2 X 6	00-00-00 00-00-00	02-04-13 00-00-04	247.65 161.01		
	2	J10 JACK-OPEN	8.00 0.00	05-10-08	05-03-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 00-05-03	35.66 21.34		
	1	J81 JACK-OPEN	6.50 0.00	03-01-08	02-03-12	2 X 4	2 X 4	01-03-08 00-00-00	00-07-07 02-03-12	10.08 7.33		
	2	J82 JACK-OPEN	6.50 0.00	03-01-08	01-07-01	2 X 4	2 X 4	01-03-08 -01-04-01	00-07-07 00-03-08	16.70 12.00		

TAMARACK LUMBER INC
ALPA LUMBER GROUP

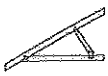
Delivery Shiplist

DATE	07/03/18
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 285494 LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH SUB-BUILDER:
MODEL: HIGHGROVE2 ELEVATION: 3-UNIT4 OR 3-UNIT 3,4

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	2	J83 JACK-OPEN	6.50 0.00	01-10-08	01-07-01	2 X 4	2 X 4	01-03-08 -00-01-01	00-07-07 00-03-08	13.84 9.34		
	5	J93 MONOPITCH	4.00 0.00	06-05-00	02-05-10	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-05-10	90.80 58.35		
	7	J110 JACK-OPEN	10.00 0.00	01-08-08	04-00-12	2 X 4	2 X 4	01-03-08 00-00-00	02-07-11 04-00-12	64.40 46.69		

TOTAL # TRUSS= **49** TOTAL BFT OF ALL TRUSSES= **1947.86** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3056.13** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
10	Hangers	LJS26DS	
1	Hangers	HGUS26-2	

TOTAL # ITEMS= **11**

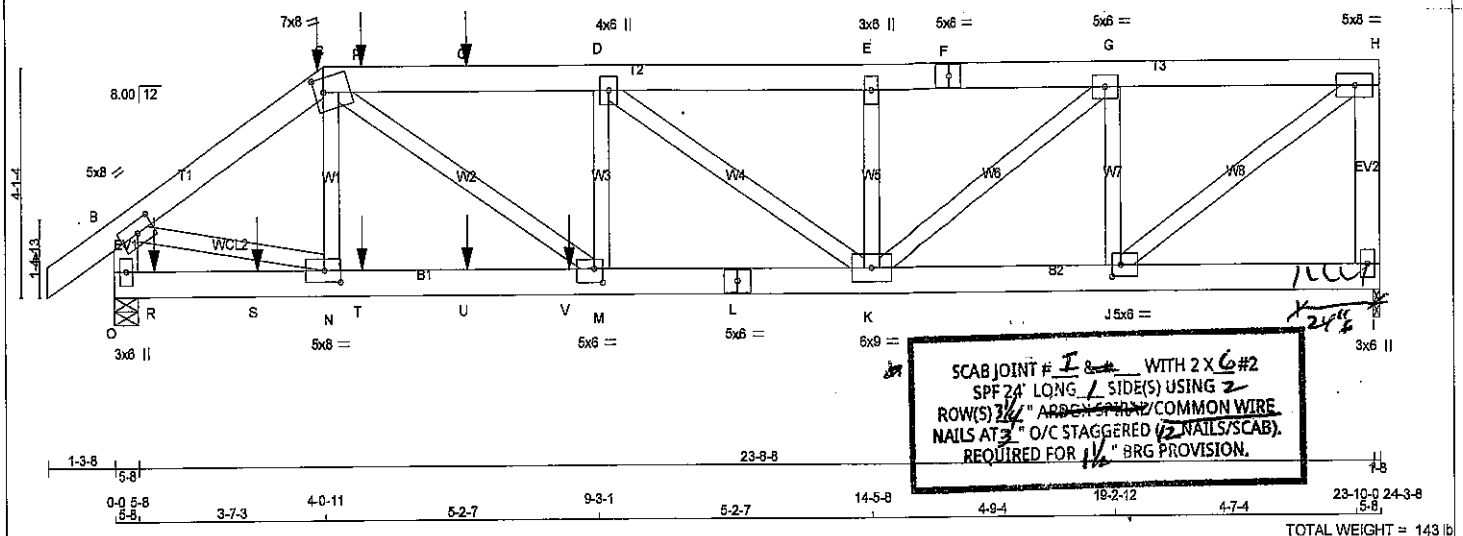
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285301	T8Z	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Wed May 16 08:35:52 2018 Page 1
ID:PFehc1Ba2cOYemTQTAYm6Oyi1EX-_W?aRIBDRIv9cIkSsFt_uHxkl5OtRMl3fvMZu8zG_tL

-1-3-8 0-0 5-8 4-0-11 9-3-1 14-5-8 19-2-12 23-10-0 24-3-8
1-3-8 5-8 3-7-3 5-2-7 5-2-7 4-9-4 4-7-4 5-8

Scale = 1:41.5



SCAB JOINT # I & ___ WITH 2 X 6 #2 SPF 24" LONG 1 SIDE(S) USING 2 ROW(S) 12" ~~ARROW STRAP~~/COMMON WIRE NAILS AT 3" O/C STAGGERED (12 NAILS/SCAB). REQUIRED FOR 1½" BRG PROVISION.

1-3-8 23-8-8 1-8
5-8

0-0 5-8 4-0-11 9-3-1 14-5-8 19-2-12 23-10-0 24-3-8
5-8 3-7-3 5-2-7 5-2-7 4-9-4 4-7-4 5-8

TOTAL WEIGHT = 143 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
I - H	2x6	DRY	No.2	SPF
O - B	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	5.0	6.0	2.50	3.75
C	TTWW-m	MT20	7.0	8.0	3.00	2.00
D	TMWW+t	MT20	4.0	6.0		
E	TMW+w	MT20	3.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMWW-t	MT20	5.0	6.0		
H	TMW-t	MT20	5.0	8.0		
I	BMV1+p	MT20	3.0	8.0		
J	BMWW-t	MT20	5.0	6.0	2.50	2.00
K	BMWWW-t	MT20	6.0	9.0		
L	BS-t	MT20	5.0	6.0		
M	BMWW-t	MT20	5.0	6.0	3.00	2.00
N	BMWW-t	MT20	5.0	8.0	2.50	3.75
O	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 244.5 lbs FACTORED DOWN AT 4-0-11, AND 113.4 lbs FACTORED DOWN AT 4-9-4, AND 101.4 lbs FACTORED DOWN AT 6-9-4 ON TOP CHORD, AND 75.5 lbs FACTORED DOWN AT 9-4, 69.9 lbs FACTORED DOWN AT 2-9-4, 69.9 lbs FACTORED DOWN AT 4-9-4, AND 69.9 lbs FACTORED DOWN AT 6-9-4, AND 1157.0 lbs FACTORED DOWN AT 8-8-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
5/16
S. KATSOULAKOS
PROVINCE OF ONTARIO

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REORD BRG IN-SX
I	1894	0	1894	0	0	1-8	1-8
O	2731	0	2731	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1548	871 / 0	345 / 0	0 / 0	0 / 0	332 / 0	0 / 0
O	2224	1265 / 0	466 / 0	0 / 0	0 / 0	473 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.73 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 33	-84.3	-84.3	0.07 (1)	10.00
B- C	-3245 / 0	-84.3	-84.3	0.19 (1)	4.53
C- P	-4458 / 0	-84.3	-84.3	0.41 (1)	3.73
P- Q	-4458 / 0	-84.3	-84.3	0.41 (1)	3.73
Q- D	-4458 / 0	-84.3	-84.3	0.41 (1)	3.73
D- E	-3629 / 0	-84.3	-84.3	0.26 (1)	4.27
E- F	-3629 / 0	-84.3	-84.3	0.22 (1)	4.30
F- G	-3629 / 0	-84.3	-84.3	0.22 (1)	4.30
G- H	-2300 / 0	-84.3	-84.3	0.18 (1)	5.21
I- H	-1833 / 0	0.0	0.0	0.27 (1)	7.35
O- B	-2675 / 0	0.0	0.0	0.19 (1)	6.33
O- R	0 / 0	-28.0	-28.0	0.19 (2)	10.00
R- S	0 / 0	-28.0	-28.0	0.19 (2)	10.00
S- N	0 / 0	-28.0	-28.0	0.19 (2)	10.00
N- T	0 / 2686	-28.0	-28.0	0.67 (1)	10.00
T- U	0 / 2686	-28.0	-28.0	0.67 (1)	10.00
U- V	0 / 2686	-28.0	-28.0	0.67 (1)	10.00
V- M	0 / 2686	-28.0	-28.0	0.67 (1)	10.00
M- L	0 / 4458	-28.0	-28.0	0.77 (1)	10.00
L- K	0 / 4458	-28.0	-28.0	0.77 (1)	10.00
K- J	0 / 2300	-28.0	-28.0	0.37 (1)	10.00
J- I	0 / 0	-28.0	-28.0	0.08 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- C	-170 / 231	0.04 (3)
C- M	0 / 2150	0.38 (1)
M- D	0 / 237	0.04 (3)
D- K	-1017 / 0	0.49 (1)
K- E	-375 / 0	0.06 (1)
K- G	0 / 1673	0.30 (1)
J- G	-1482 / 0	0.25 (1)
J- H	0 / 2833	0.50 (1)
B- N	0 / 2753	0.49 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	4-0-11	-23	-25	—	FRONT	VERT	DEAD
C	4-0-11	-222	-222	—	FRONT	VERT	SNOW
P	4-9-4	-113	-113	—	FRONT	VERT	TOTAL
Q	6-9-4	-101	-101	—	FRONT	VERT	TOTAL
R	9-4	-43	-76	—	FRONT	VERT	TOTAL
S	2-9-4	-40	-70	—	FRONT	VERT	TOTAL
T	4-9-4	-40	-70	—	FRONT	VERT	TOTAL
U	6-9-4	-40	-70	—	FRONT	VERT	TOTAL
V	8-8-8	-1157	-1157	—	FRONT	VERT	TOTAL

[M]

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.41/1.00 (C-D:1) , BC=0.77/1.00 (K-M:1) , WB=0.50/1.00 (H-J:1) , SSI=0.74/1.00 (M-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1687	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.65 (L) (INPUT = 1.00)

DWG NO. TAM 26663-8
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43994	DRWG NO.
285813	T8Z4	1	1	TRUSS DESC.	

Temarack Roof Truss, Burlington

Version 8.200 S Dec 12 2017 MiTek Industries, Inc. Tue Jan 23 08:16:43 2018 Page 1
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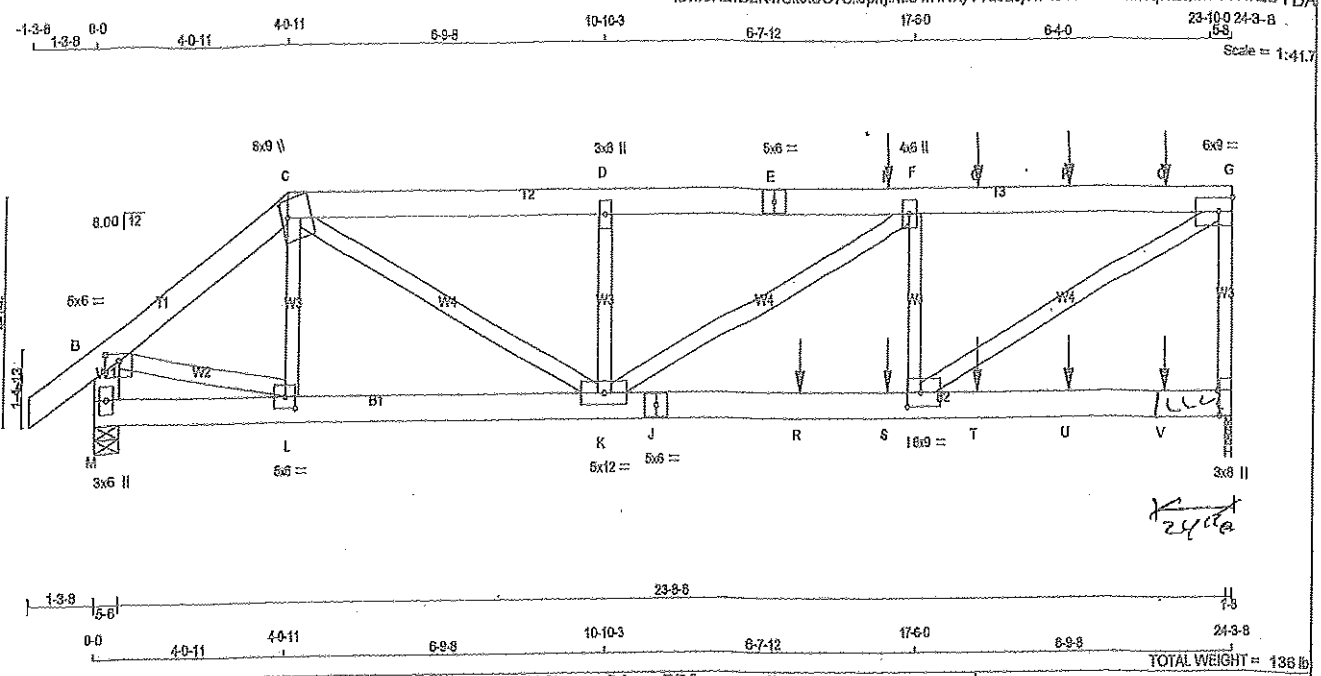


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - G	2x6	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x6	DRY	No.2	SPF
M - J	2x6	DRY	2100F 1.8E	SPF
J - H	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW-p	MT20	5.0	8.0	1.50	3.00
C	TTWW+m	MT20	8.0	9.0	3.75	1.75
D	TMW+w	MT20	3.0	6.0		
E	TS-t	MT20	5.0	6.0		
F	TMWW+t	MT20	4.0	6.0		
G	TMW-t	MT20	6.0	9.0		Edge
H	BMV1+p	MT20	3.0	8.0	5.50	0.50
I	BMWW-t	MT20	6.0	9.0	3.00	3.75
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	5.0	12.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.75
M	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 101.4 lbs FACTORED DOWN AT 16-10-12, 101.4 lbs FACTORED DOWN AT 18-10-12, AND 101.4 lbs FACTORED DOWN AT 20-10-12, AND 101.4 lbs FACTORED DOWN AT 22-10-12 ON TOP CHORD, AND 1340.4 lbs FACTORED DOWN AT 14-11-8, 69.9 lbs FACTORED DOWN AT 16-10-12, 69.9 lbs FACTORED DOWN AT 18-10-12, AND 69.9 lbs FACTORED DOWN AT 20-10-12, AND 69.9 lbs FACTORED DOWN AT 22-10-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

SCAB JOINT # H & ~~I~~ WITH 2 X 6 #2 SPF 24" LONG / SIDE(S) USING 2 ROW(S) 3¼" ARDOX SPIRAL/~~COMMON WIRE~~ NAILS AT 2" O/C STAGGERED (12 NAILS/SCAB). REQUIRED FOR 1/4 SRG PROVISION.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	2652	0	2652	0	0	1-8	4.25
M	2100	0	2100	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	2178	1209 / 0	486 / 0	0 / 0	0 / 0	473 / 0	0 / 0
M	1704	979 / 0	366 / 0	0 / 0	0 / 0	359 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.61 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 33	-84.3	-84.3	0.07 (1)	10.00
B-C	-2365 / 0	-84.3	-84.3	0.16 (1)	5.17
C-D	-4311 / 0	-84.3	-84.3	0.43 (1)	3.79
D-E	-4311 / 0	-84.3	-84.3	0.51 (1)	3.66
E-N	-4311 / 0	-84.3	-84.3	0.51 (1)	3.66
N-F	-4311 / 0	-84.3	-84.3	0.51 (1)	3.66
F-O	-4173 / 0	-84.3	-84.3	0.59 (1)	3.61
O-P	-4173 / 0	-84.3	-84.3	0.59 (1)	3.61
P-Q	-4173 / 0	-84.3	-84.3	0.59 (1)	3.61
Q-G	-4173 / 0	-84.3	-84.3	0.59 (1)	3.61
H-G	-2699 / 0	0.0	0.0	0.60 (1)	5.26
M-B	-2029 / 0	0.0	0.0	0.14 (1)	7.06
M-L	0 / 0	-28.0	-28.0	0.04 (1)	10.00
L-K	0 / 1946	-28.0	-28.0	0.33 (1)	10.00
K-J	0 / 4173	-28.0	-28.0	0.65 (1)	10.00
J-R	0 / 4173	-28.0	-28.0	0.65 (1)	10.00
R-S	0 / 4173	-28.0	-28.0	0.65 (1)	10.00
S-I	0 / 4173	-28.0	-28.0	0.65 (1)	10.00
I-T	0 / 0	-28.0	-28.0	0.31 (1)	10.00
T-U	0 / 0	-28.0	-28.0	0.31 (1)	10.00
U-V	0 / 0	-28.0	-28.0	0.31 (1)	10.00
V-H	0 / 0	-28.0	-28.0	0.31 (1)	10.00

WEBS

MEMB. FR-TO	FORCE (LBS)	MAX CSI (LC)
L-C	-373 / 26	0.05 (1)
C-K	0 / 2678	0.47 (1)
K-D	-571 / 0	0.10 (1)
K-F	0 / 157	0.03 (1)
I-F	-994 / 0	0.17 (1)
I-G	0 / 4736	0.84 (1)
B-L	0 / 2097	0.35 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
N	16-10-12	-101	-101	--	FRONT	VERT	TOTAL
O	18-10-12	-101	-101	--	FRONT	VERT	TOTAL
P	20-10-12	-101	-101	--	FRONT	VERT	TOTAL
Q	22-10-12	-101	-101	--	FRONT	VERT	TOTAL
R	14-11-8	-1340	-1340	--	FRONT	VERT	TOTAL
S	16-10-12	-40	-70	--	FRONT	VERT	TOTAL
T	18-10-12	-40	-70	--	FRONT	VERT	TOTAL
U	20-10-12	-40	-70	--	FRONT	VERT	TOTAL
V	22-10-12	-40	-70	--	FRONT	VERT	TOTAL

DESIGN CRITERIA

[M]

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 063-09
- TPIC 2011

(55 % OF 31.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.27")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 684 (0.43")

CSI: TC=0.60/1.00 (G-H:1) , BC=0.86/1.00 (I-K:1) , WB=0.84/1.00 (G-I:1) , SSI=0.69/1.00 (I-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

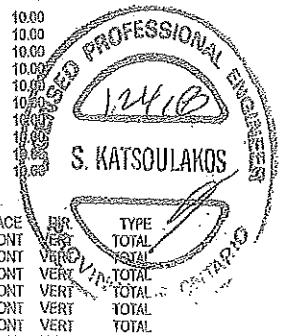
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1658

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.68 (i) (INPUT = 0.90)
JSI METAL= 0.70 (i) (INPUT = 1.00)



DWG NO. TAM5069-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285994	T9	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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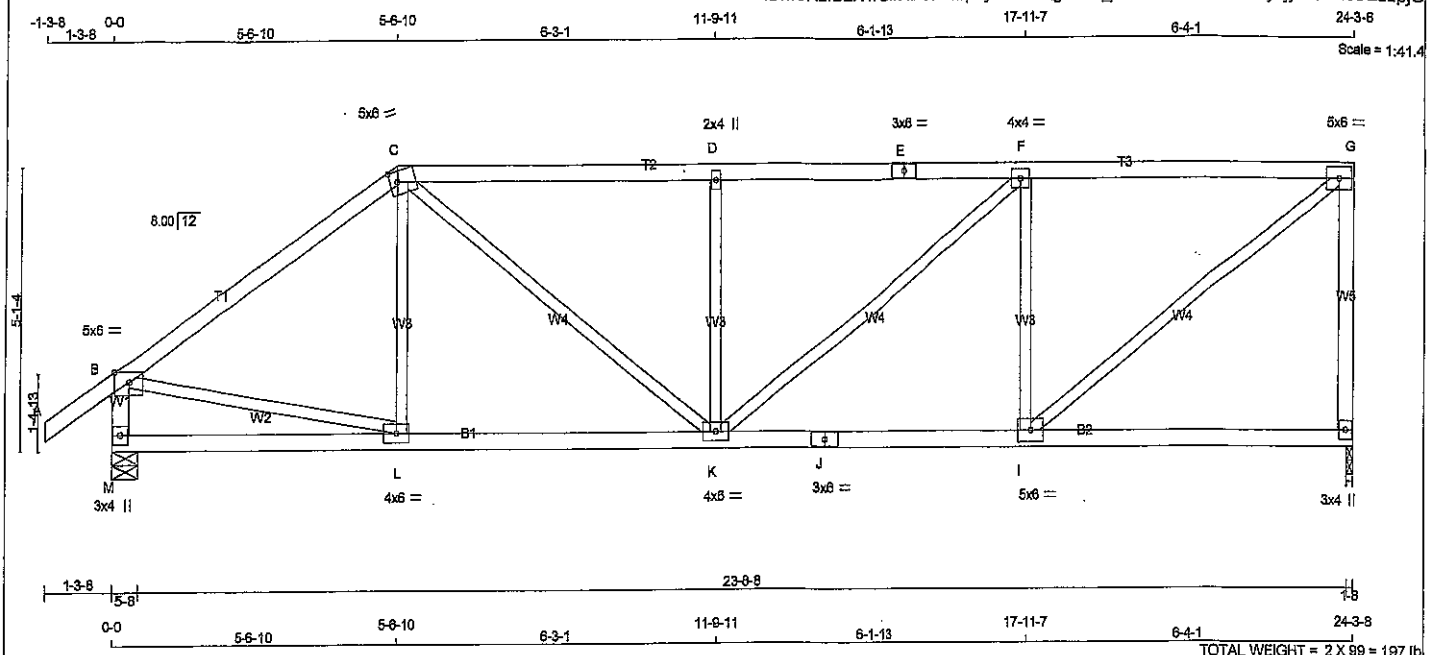

TOTAL WEIGHT = 2 X 99 = 197 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TTWW-m	MT20	5.0	6.0	2.25	1.75
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H	1363	0	1363	0	0	1-8	1-8
M	1479	0	1479	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1119	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
M	1198	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.34 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	L- C	-49 / 204	0.05 (3)
B- C	-1493 / 0	-84.3	-84.3	0.55 (1)	4.69	B- L	0 / 1263	0.28 (1)
C- D	-1797 / 0	-84.3	-84.3	0.55 (1)	4.37	I- G	0 / 1778	0.40 (1)
D- E	-1797 / 0	-84.3	-84.3	0.56 (1)	4.34	C- K	0 / 702	0.16 (1)
E- F	-1797 / 0	-84.3	-84.3	0.56 (1)	4.34	I- F	-877 / 0	0.34 (1)
F- G	-1416 / 0	-84.3	-84.3	0.53 (1)	4.81	K- D	-566 / 0	0.22 (1)
H- G	-1292 / 0	0.0	0.0	0.57 (1)	7.10	K- F	0 / 484	0.11 (1)
M- B	-1417 / 0	0.0	0.0	0.15 (1)	6.85			
M- L	0 / 0	-28.0	-28.0	0.24 (3)	10.00			
L- K	0 / 1240	-28.0	-28.0	0.39 (2)	10.00			
K- J	0 / 1416	-28.0	-28.0	0.44 (2)	10.00			
J- I	0 / 1416	-28.0	-28.0	0.44 (2)	10.00			
I- H	0 / 0	-28.0	-28.0	0.27 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.57/1.00 (G-H:1) , BC=0.44/1.00 (I-K:2) , WB=0.40/1.00 (G-I:1) , SSI=0.25/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (K) (INPUT = 0.90)
JSI METAL= 0.44 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
1168
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 3988-19
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285994	T10	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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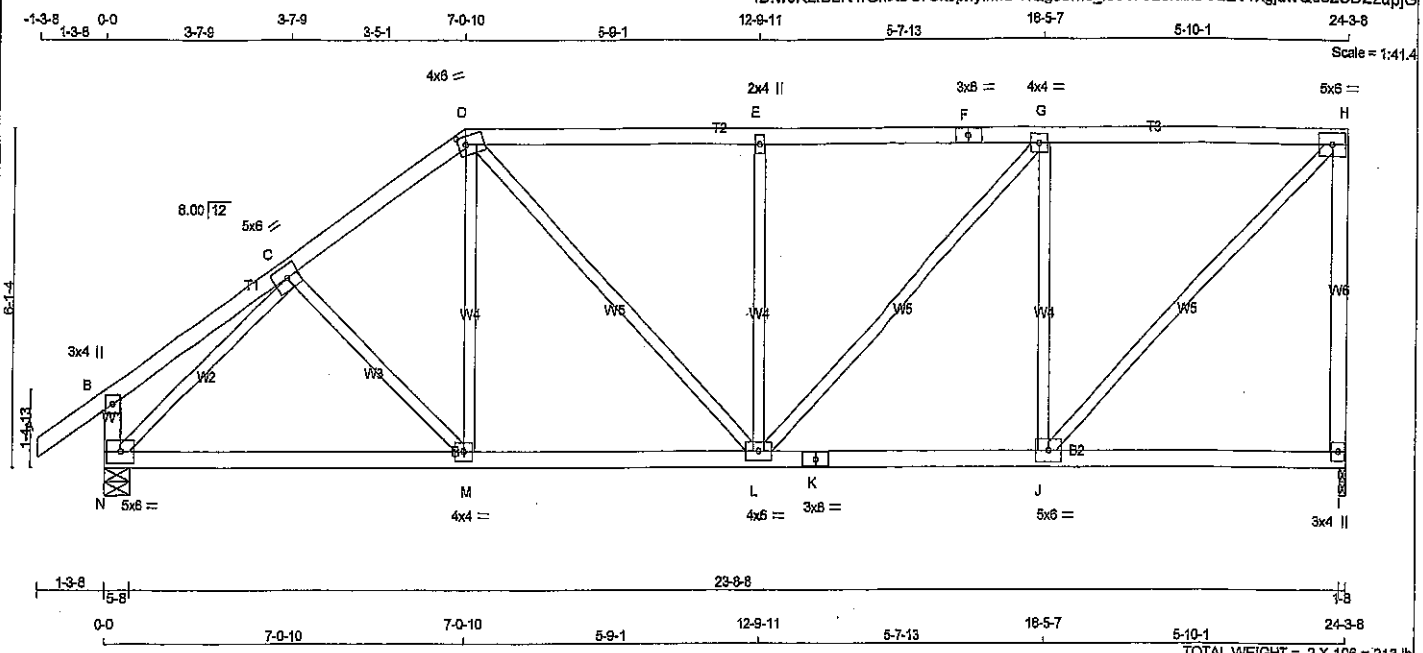

TOTAL WEIGHT = 2 X 106 = 213 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW-m	MT20	4.0	6.0	1.75	1.75
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TMVW-t	MT20	5.0	6.0		
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	5.0	6.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	4.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	1363	0	1363	0	0	1-8	1-8
N	1479	0	1479	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1119	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
N	1198	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.86 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	(LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.3	-84.3	0.11	(1)		10.00
B- C	0 / 17	-84.3	-84.3	0.15	(1)		10.00
C- D	-1460 / 0	-84.3	-84.3	0.18	(1)		5.22
D- E	-1473 / 0	-84.3	-84.3	0.43	(1)		4.66
E- F	-1473 / 0	-84.3	-84.3	0.44	(1)		4.86
F- G	-1473 / 0	-84.3	-84.3	0.44	(1)		4.86
G- H	-1103 / 0	-84.3	-84.3	0.42	(1)		5.46
I- H	-1298 / 0	0.0	0.0	0.94	(1)		7.06
N- B	-233 / 0	0.0	0.0	0.02	(1)		7.81
N- M	0 / 1196	-28.0	-28.0	0.45	(2)		10.00
M- L	0 / 1202	-28.0	-28.0	0.47	(2)		10.00
L- K	0 / 1103	-28.0	-28.0	0.37	(2)		10.00
K- J	0 / 1103	-28.0	-28.0	0.37	(2)		10.00
J- I	0 / 0	-28.0	-28.0	0.24	(3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI	(LC)
C- M	0 / 118	0.03	(3)
M- D	0 / 307	0.07	(2)
N- C	-1669 / 0	0.64	(1)
J- H	0 / 1557	0.35	(1)
D- L	0 / 385	0.09	(1)
J- G	-912 / 0	0.53	(1)
L- E	-520 / 0	0.30	(1)
L- G	0 / 530	0.12	(1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.8	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- C&A 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.94/1.00 (H-I:1) , BC=0.47/1.00 (L-M:2) , WB=0.64/1.00 (C-N:1) , SSI=0.23/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.60 (D) (INPUT = 0.90)
JSI METAL= 0.41 (C) (INPUT = 1.00)

DWG NO. TAM 3989 -18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285994	T11	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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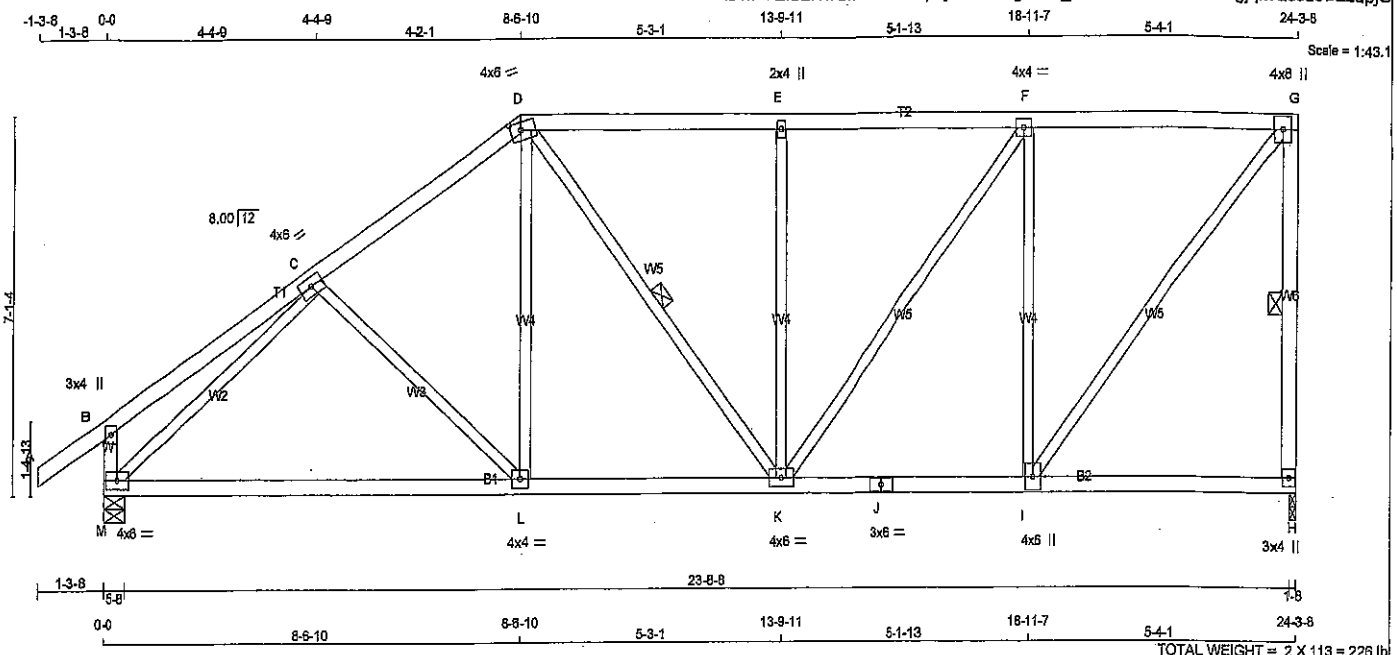


TOTAL WEIGHT = 2 X 113 = 226 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW-m	MT20	4.0	6.0	1.75	2.50
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW+p	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWW+t	MT20	4.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQR'D BRG
H	1363	0	1363	0	0	1-8	1-8
M	1479	0	1479	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1119	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
M	1198	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

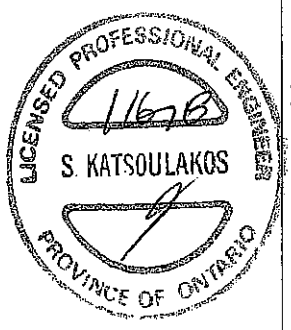
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-H, D-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	C- L	-117 / 85	0.07 (1)
B- C	0 / 23	-84.3	-84.3	0.24 (1)	10.00	L- D	0 / 414	0.09 (2)
C- D	-1390 / 0	-84.3	-84.3	0.28 (1)	5.20	M- C	-1661 / 0	0.93 (1)
D- E	-1226 / 0	-84.3	-84.3	0.34 (1)	5.37	I- G	0 / 1426	0.32 (1)
E- F	-1227 / 0	-84.3	-84.3	0.35 (1)	5.35	D- K	-6 / 141	0.03 (1)
F- G	-879 / 0	-84.3	-84.3	0.34 (1)	6.07	I- F	-948 / 0	0.83 (1)
H- G	-1305 / 0	0.0	0.0	0.30 (1)	5.65	K- E	-474 / 0	0.41 (1)
M- B	-255 / 0	0.0	0.0	0.03 (1)	7.81	K- F	0 / 576	0.13 (1)
M- L	0 / 1223	-28.0	-28.0	0.59 (2)	10.00			
L- K	0 / 1141	-28.0	-28.0	0.59 (2)	10.00			
K- J	0 / 879	-28.0	-28.0	0.31 (2)	10.00			
J- I	0 / 879	-28.0	-28.0	0.31 (2)	10.00			
I- H	0 / 0	-28.0	-28.0	0.21 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.23")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 779 (0.37")

CSI: TC=0.35/1.00 (E-F:1) , BC=0.59/1.00 (K-L:2) , WB=0.93/1.00 (C-M:1) , SSI=0.21/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	622	2284	1655

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (M) (INPUT = 0.90)
JSI METAL= 0.40 (C) (INPUT = 1.00)

DWG NO. TAM 3990 -18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285994	T12	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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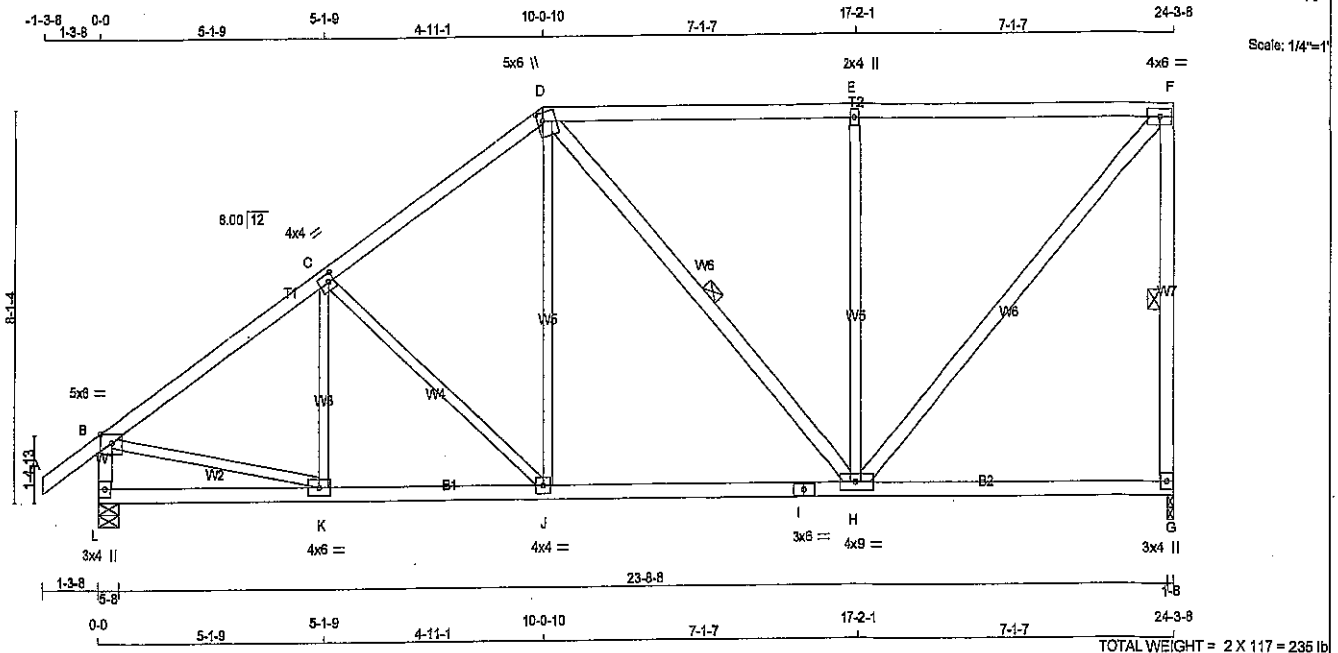

TOTAL WEIGHT = 2 X 117 = 235 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1363	0	1363	0	0	1-8	1-8
L	1479	0	1479	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1119	821 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
L	1198	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.81 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00
B- C	-1529 / 0	-84.3	-84.3	0.43 (1)	4.83
C- D	-1275 / 0	-84.3	-84.3	0.40 (1)	5.20
D- E	-967 / 0	-84.3	-84.3	0.80 (1)	4.81
E- F	-967 / 0	-84.3	-84.3	0.81 (1)	4.81
G- F	-1284 / 0	0.0	0.0	0.36 (1)	5.69
L- B	-1418 / 0	0.0	0.0	0.15 (1)	6.85
L- K	0 / 0	-28.0	-28.0	0.16 (3)	10.00
K- J	0 / 1295	-28.0	-28.0	0.33 (2)	10.00
J- I	0 / 1041	-28.0	-28.0	0.49 (2)	10.00
I- H	0 / 1041	-28.0	-28.0	0.49 (2)	10.00
H- G	0 / 0	-28.0	-28.0	0.37 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-123 / 133	0.04 (1)
C- J	-347 / 0	0.29 (1)
J- D	0 / 436	0.10 (2)
D- H	-110 / 0	0.06 (1)
H- E	-745 / 0	0.94 (1)
H- F	0 / 1432	0.23 (1)
B- K	0 / 1323	0.30 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(65 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.81/1.00 (E-F:1) , BC=0.49/1.00 (H-J:2) , WB=0.94/1.00 (E-H:1) , SSI=0.29/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL= 0.36 (I) (INPUT = 1.00)

DWG NO. TAM 3991-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
286039	T13	5	1	TRUSS DESC.	

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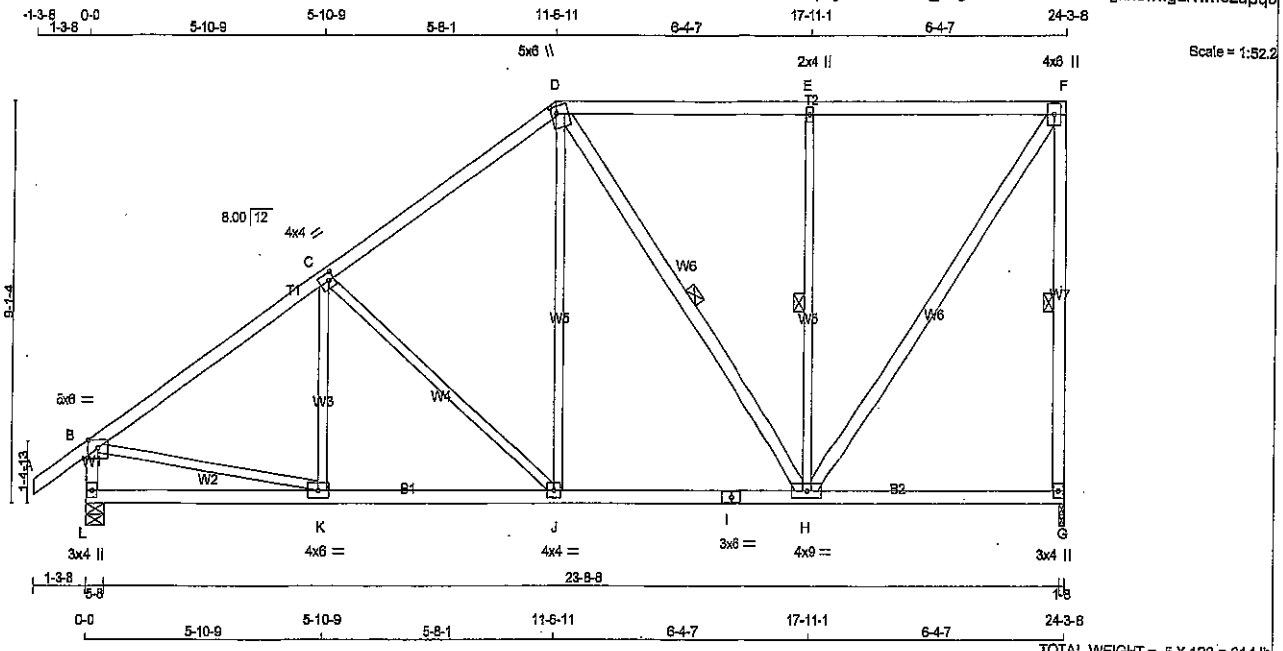


TOTAL WEIGHT = 5 X 123 = 614 lb (M)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table Js in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQR0 BRG IN-SX
G	1363	0	1363	0	0	1-8	1-8
L	1479	0	1479	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1119	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
L	1198	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.61 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

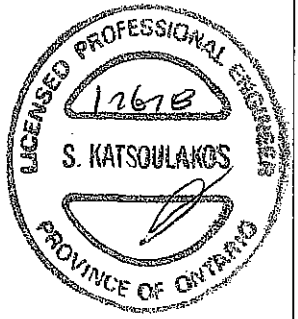
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	K- C	-56 / 203	0.05 (3)
B- C	-1531 / 0	-84.3	-84.3	0.59 (1)	4.61	C- J	-474 / 0	0.56 (1)
C- D	-1167 / 0	-84.3	-84.3	0.54 (1)	5.15	J- D	0 / 490	0.11 (2)
D- E	-790 / 0	-84.3	-84.3	0.61 (1)	5.73	D- H	-270 / 0	0.17 (1)
E- F	-790 / 0	-84.3	-84.3	0.62 (1)	5.73	H- E	-666 / 0	0.36 (1)
G- F	-1292 / 0	0.0	0.0	0.50 (1)	5.68	H- F	0 / 1344	0.22 (1)
L- B	-1412 / 0	0.0	0.0	0.15 (1)	6.66	B- K	0 / 1322	0.30 (1)
L- K	0 / 0	-26.0	-26.0	0.23 (3)	10.00			
K- J	0 / 1300	-26.0	-26.0	0.36 (2)	10.00			
J- I	0 / 948	-26.0	-26.0	0.40 (2)	10.00			
I- H	0 / 948	-26.0	-26.0	0.40 (2)	10.00			
H- G	0 / 0	-26.0	-26.0	0.29 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.62/1.00 (E-F:1) , BC=0.40/1.00 (H-J:2) , WB=0.56/1.00 (C-J:1) , SSI=0.28/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.32 (K) (INPUT = 1.00)

DWG NO. TAM 3993 -13
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285746	T14	6	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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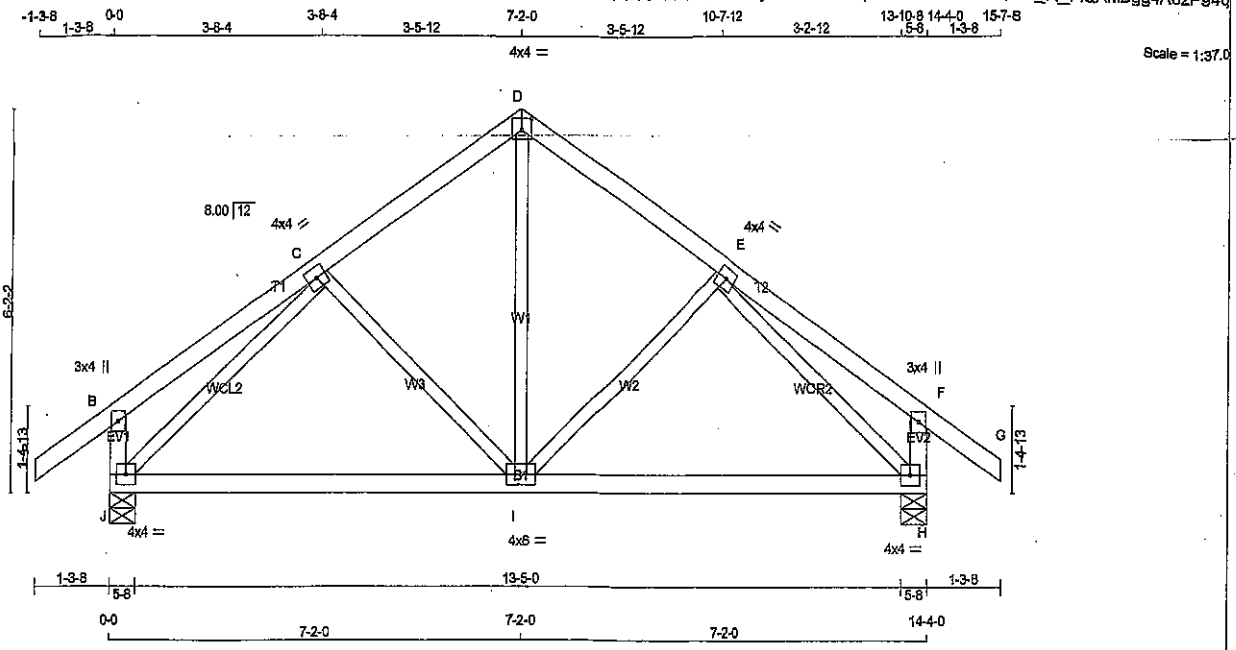

TOTAL WEIGHT = 6 X 62 = 371 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	920	0	920	0	0	5-8	5-8
H	920	0	920	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	739	437 / 0	151 / 0	0 / 0	0 / 0	152 / 0	0 / 0
H	739	437 / 0	151 / 0	0 / 0	0 / 0	152 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	I- D	0 / 460	0.10 (1)	
B- C	0 / 19	-84.3	-84.3	0.17 (1)	10.00	I- E	-152 / 44	0.06 (1)	
C- D	-645 / 0	-84.3	-84.3	0.13 (1)	6.25	C- I	-152 / 44	0.06 (1)	
D- E	-645 / 0	-84.3	-84.3	0.13 (1)	6.25	J- C	-879 / 0	0.35 (1)	
E- F	0 / 19	-84.3	-84.3	0.17 (1)	10.00	E- H	-879 / 0	0.35 (1)	
F- G	0 / 32	-84.3	-84.3	0.11 (1)	10.00				
J- B	-232 / 0	0.0	0.0	0.02 (1)	7.81				
H- F	-232 / 0	0.0	0.0	0.02 (1)	7.81				
J- I	0 / 631	-28.0	-28.0	0.49 (2)	10.00				
I- H	0 / 631	-28.0	-28.0	0.49 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT. CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	48.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.48")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.48")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.17/1.00 (E-F:1) , BC=0.49/1.00 (H-I:2) , WB=0.35/1.00 (E-H:1) , SSI=0.17/1.00 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	922	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (D) (INPUT = 0.90)
JSI METAL= 0.31 (C) (INPUT = 1.00)

DWG NO. TAM 2126 -18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285876	T15A	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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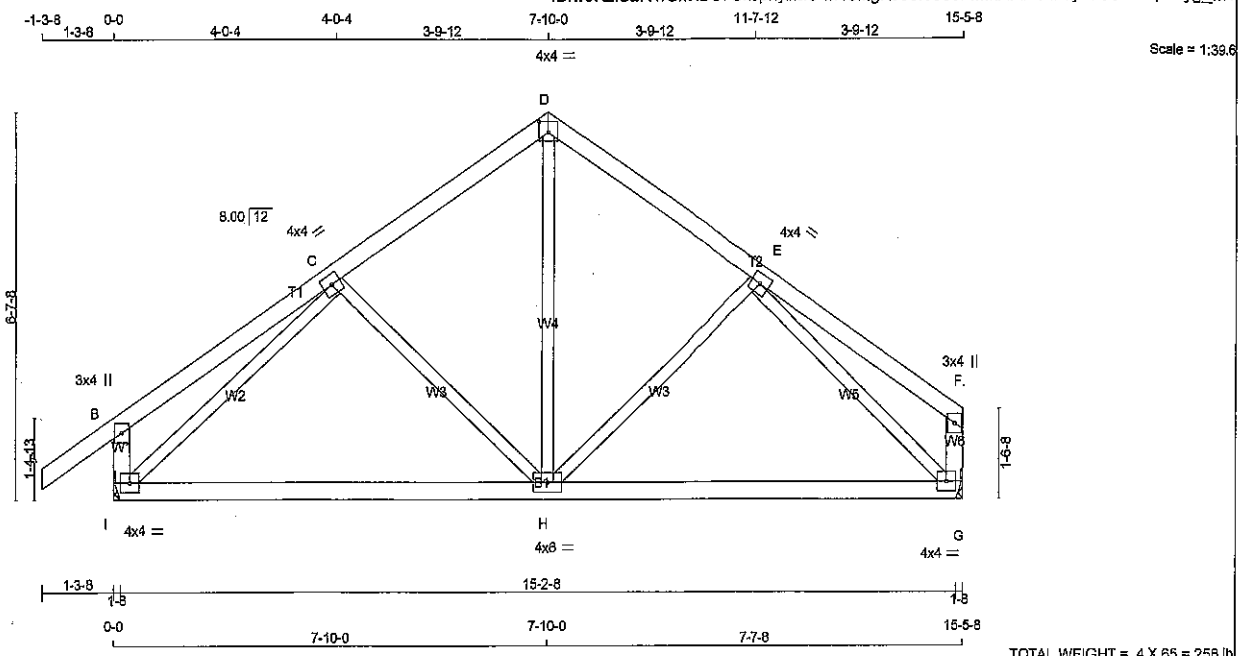

TOTAL WEIGHT = 4 X 65 = 256 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BMWWW-t	MT20	4.0	6.0		
I	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
I	983	0	983	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
G	868	0	868	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	791	468 / 0	162 / 0	0 / 0	0 / 0	163 / 0	0 / 0
G	712	395 / 0	162 / 0	0 / 0	0 / 0	155 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00
B- C	0 / 21	-84.3	-84.3	0.20 (1)	10.00
C- D	-698 / 0	-84.3	-84.3	0.16 (1)	6.25
D- E	-697 / 0	-84.3	-84.3	0.15 (1)	6.25
E- F	0 / 21	-84.3	-84.3	0.19 (1)	10.00
I- B	-242 / 0	0.0	0.0	0.02 (1)	7.81
G- F	-118 / 0	0.0	0.0	0.01 (1)	7.81
I- H	0 / 696	-28.0	-28.0	0.56 (2)	10.00
H- G	0 / 672	-28.0	-28.0	0.56 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- D	0 / 494	0.11 (1)
H- E	-148 / 53	0.07 (1)
C- H	-182 / 41	0.09 (1)
I- C	-957 / 0	0.45 (1)
E- G	-948 / 0	0.42 (1)

DWG NO. TAM 45029-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.52")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.52")
CALCULATED VERT. DEFL.(TL) = L/ 981 (0.19")

CSI: TC=0.20 (B-C:1) , BC=0.56 (H-I:2) , WB=0.45 (C-I:1) , SSI=0.19 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (D) (INPUT = 0.90)
JSI METAL= 0.34 (C) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285876	G15A	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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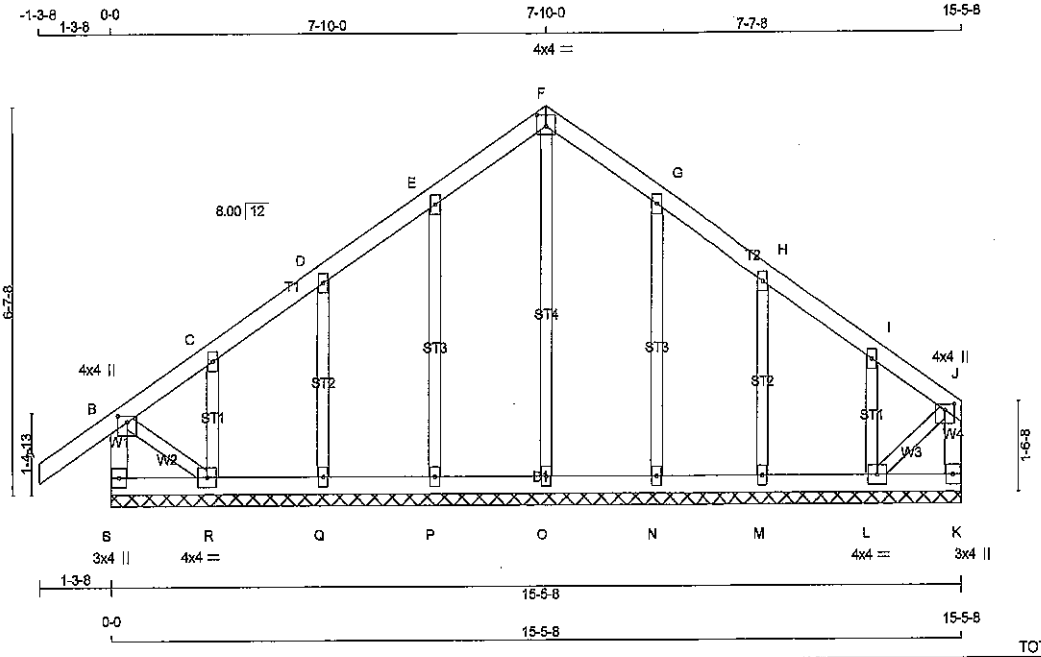

TOTAL WEIGHT = 2 X 68 = 135 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
S - B	2x4	DRY	No.2	SPF
A - F	2x4	DRY	No.2	SPF
F - J	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
S - K	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C, D, E, G, H, I						
C	TMW+w	MT20	2.0	4.0		
F	TTW-p	MT20	4.0	4.0	2.25	2.00
J	TMVW+p	MT20	4.0	4.0	1.25	2.00
K	BMV1+p	MT20	3.0	4.0		
L	BMWW1-t	MT20	4.0	4.0		
M, N, O, P, Q						
M	BMW1+w	MT20	2.0	4.0		
R	BMWW1-t	MT20	4.0	4.0		
S	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
S- B	-256 / 0	0.0	0.0	0.03 (1)	7.81
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00
B- C	-50 / 0	-84.3	-84.3	0.11 (1)	6.25
C- D	-19 / 0	-84.3	-84.3	0.04 (1)	6.25
D- E	-18 / 0	-84.3	-84.3	0.05 (1)	6.25
E- F	-25 / 0	-84.3	-84.3	0.05 (1)	6.25
F- G	-25 / 0	-84.3	-84.3	0.05 (1)	6.25
G- H	-17 / 0	-84.3	-84.3	0.05 (1)	6.25
H- I	-24 / 0	-84.3	-84.3	0.04 (1)	6.25
I- J	-21 / 0	-84.3	-84.3	0.04 (1)	6.25
K- J	-84 / 0	0.0	0.0	0.01 (1)	7.81
S- R	0 / 0	-28.0	-28.0	0.03 (2)	10.00
R- Q	0 / 20	-28.0	-28.0	0.03 (2)	10.00
Q- P	0 / 16	-28.0	-28.0	0.02 (2)	10.00
P- O	0 / 13	-28.0	-28.0	0.02 (2)	10.00
O- N	0 / 13	-28.0	-28.0	0.02 (2)	10.00
N- M	0 / 16	-28.0	-28.0	0.02 (2)	10.00
M- L	0 / 20	-28.0	-28.0	0.02 (2)	10.00
L- K	0 / 0	-28.0	-28.0	0.02 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- F	-124 / 0	0.09 (1)
P- E	-189 / 0	0.08 (1)
Q- D	-172 / 0	0.04 (1)
R- C	-108 / 0	0.02 (1)
N- G	-191 / 0	0.08 (1)
M- H	-162 / 0	0.04 (1)
L- I	-152 / 0	0.03 (1)
B- R	0 / 28	0.01 (1)
L- J	0 / 32	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11 (A-B:1) , BC=0.03 (Q-R:2) , WB=0.09 (F-O:1) , SSI=0.08 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (B) (INPUT = 0.90)
JSI METAL= 0.05 (G) (INPUT = 1.00)

DWG NO. TAM45066-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285876	T24	20	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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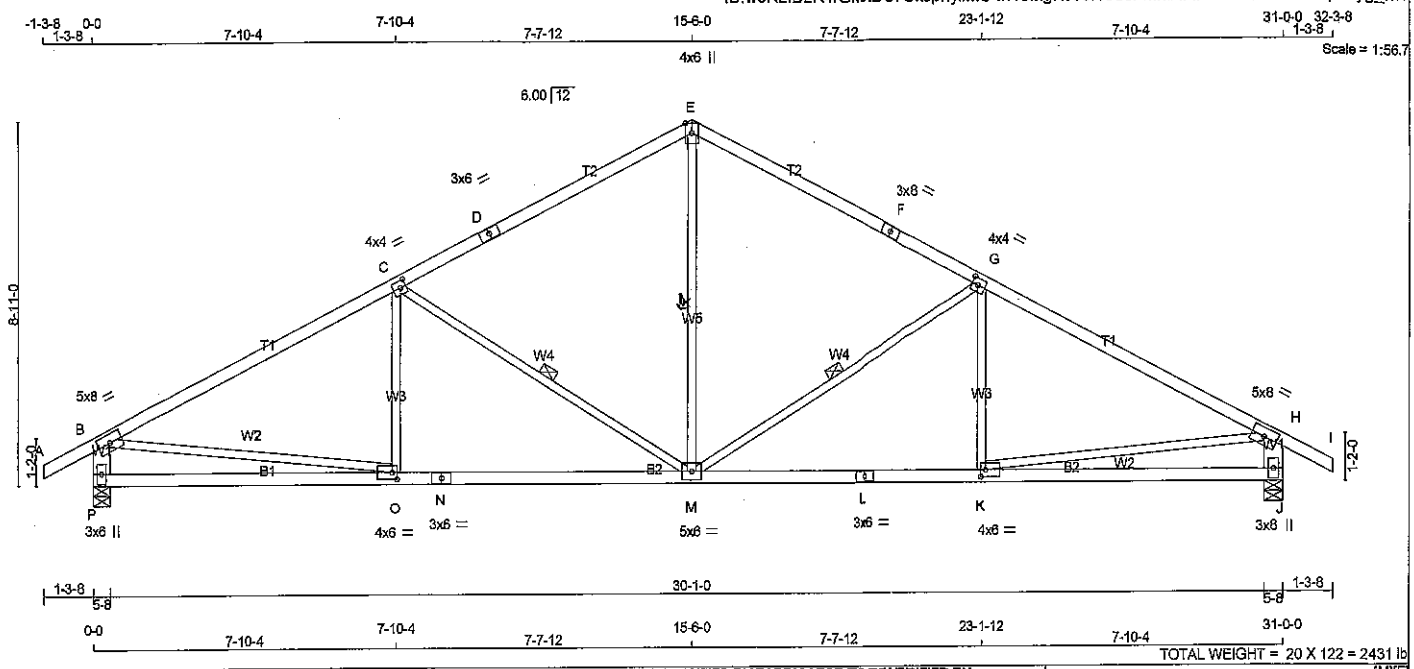


TOTAL WEIGHT = 20 X 122 = 2431 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
P - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	6.0		
E	TTW+p	MT20	4.0	6.0	Edge	
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
H	TMVW-t	MT20	5.0	8.0		
J	BMV1+p	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	8.0	2.00	1.50
L	BS-t	MT20	3.0	8.0		
M	BMWWW-t	MT20	5.0	6.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	6.0	2.00	1.50
P	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	1854	0	1854	0	0	5-8	5-8
J	1854	0	1854	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1506	863 / 0	326 / 0	0 / 0	0 / 0	318 / 0	0 / 0
J	1508	863 / 0	326 / 0	0 / 0	0 / 0	318 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.27 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

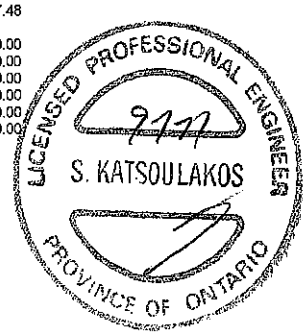
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-M, C-M. EMP

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF)	CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 26	-84.3	-84.3	0.11 (1)	10.00	
B- C	-2529 / 0	-84.3	-84.3	0.94 (1)	3.27	
C- D	-1803 / 0	-84.3	-84.3	0.78 (1)	3.96	
D- E	-1803 / 0	-84.3	-84.3	0.78 (1)	3.96	
E- F	-1803 / 0	-84.3	-84.3	0.78 (1)	3.96	
F- G	-1803 / 0	-84.3	-84.3	0.78 (1)	3.96	
G- H	-2529 / 0	-84.3	-84.3	0.94 (1)	3.27	
H- I	0 / 26	-84.3	-84.3	0.11 (1)	10.00	
P- B	-1764 / 0	0.0	0.0	0.11 (1)	7.48	
J- H	-1764 / 0	0.0	0.0	0.11 (1)	7.48	
P- O	0 / 0	-28.0	-28.0	0.42 (3)	10.00	
O- N	0 / 2293	-28.0	-28.0	0.68 (2)	10.00	
N- M	0 / 2293	-28.0	-28.0	0.68 (2)	10.00	
M- L	0 / 2293	-28.0	-28.0	0.68 (2)	10.00	
L- K	0 / 2293	-28.0	-28.0	0.68 (2)	10.00	
K- J	0 / 0	-28.0	-28.0	0.42 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- E	0 / 1096	0.25 (1)
M- G	-840 / 0	0.48 (1)
K- G	-9 / 307	0.07 (3)
C- M	-840 / 0	0.48 (1)
O- C	-9 / 307	0.07 (3)
B- O	0 / 2306	0.52 (1)
K- H	0 / 2306	0.52 (1)



DWG NO. TAM 45030-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.30")

CSI: TC=0.94 (G-H:1) , BC=0.68 (K-M:2) , WB=0.52 (H-K:1) , SSI=0.29 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (K) (INPUT = 0.90)
JSI METAL= 0.69 (L) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285864	T30	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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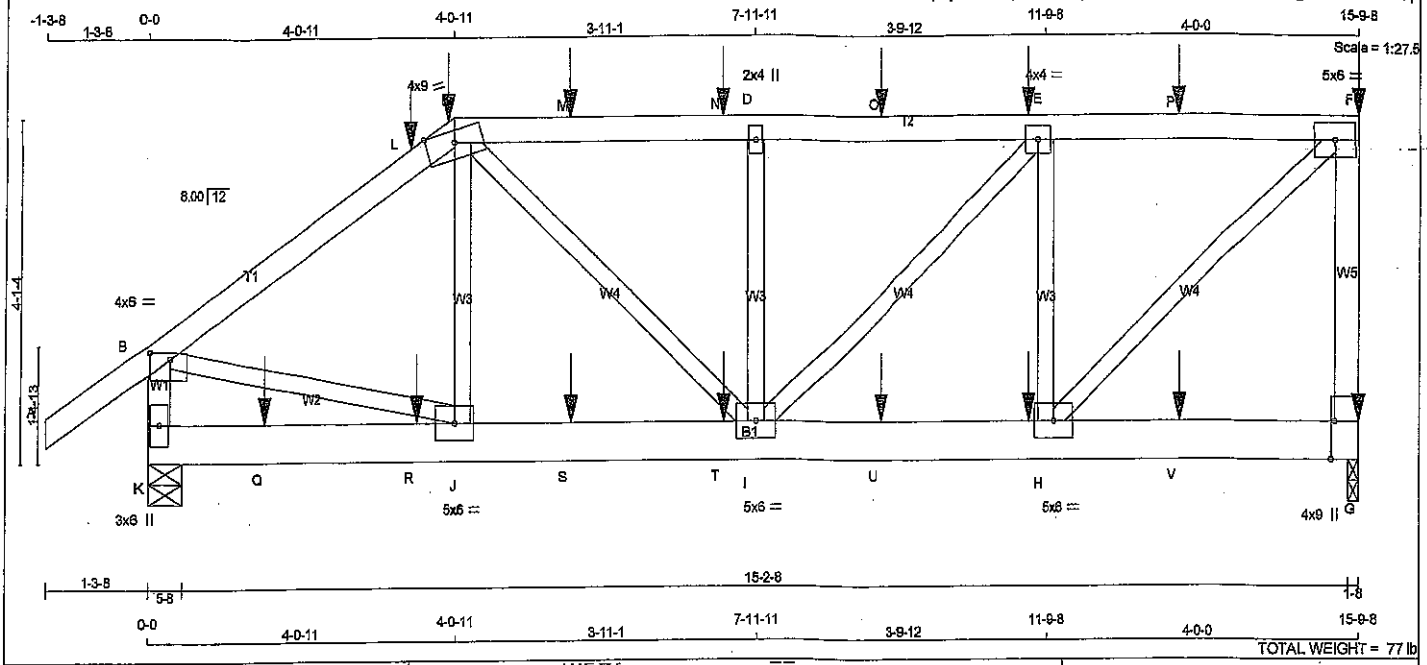


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - G	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	6.0	1.00	3.25
C	TTWW-m	MT20	4.0	9.0	Edge	
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMVW-t	MT20	5.0	6.0		
G	BMV1+p	MT20	4.0	9.0	Edge	0.50
H	BMWW-t	MT20	5.0	6.0		
I	BMWWW-t	MT20	5.0	6.0		
J	BMWW-t	MT20	5.0	6.0		
K	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 244.5 lbs FACTORED DOWN AT 4-0-11, 122.8 lbs FACTORED DOWN AT 3-6-12, 101.4 lbs FACTORED DOWN AT 5-6-12, 101.4 lbs FACTORED DOWN AT 7-6-12, 101.4 lbs FACTORED DOWN AT 9-6-12, 101.4 lbs FACTORED DOWN AT 11-6-12, AND 101.4 lbs FACTORED DOWN AT 13-6-12, AND 133.9 lbs FACTORED DOWN AT 15-9-8 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 1-6-12, 69.9 lbs FACTORED DOWN AT 3-6-12, 69.9 lbs FACTORED DOWN AT 5-6-12, 69.9 lbs FACTORED DOWN AT 7-6-12, 69.9 lbs FACTORED DOWN AT 9-6-12, 69.9 lbs FACTORED DOWN AT 11-6-12, AND 69.9 lbs FACTORED DOWN AT 13-6-12, AND 68.8 lbs FACTORED DOWN AT 15-9-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1604	0	1604	0	0	1-8	1-8
K	1622	0	1622	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1321	726 / 0	305 / 0	0 / 0	0 / 0	289 / 0	0 / 0
K	1310	763 / 0	275 / 0	0 / 0	0 / 0	273 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.69 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 32	-84.3	-84.3	0.12 (1)	10.00	J- C	-105 / 224	0.06 (3)
B- L	-1622 / 0	-84.3	-84.3	0.37 (1)	4.74	B- J	0 / 1407	0.35 (1)
L- C	-1622 / 0	-84.3	-84.3	0.37 (1)	4.74	H- F	0 / 1736	0.43 (1)
C- M	-1683 / 0	-84.3	-84.3	0.34 (1)	4.69	C- I	0 / 437	0.11 (1)
M- N	-1683 / 0	-84.3	-84.3	0.34 (1)	4.69	H- E	-974 / 0	0.25 (1)
N- D	-1683 / 0	-84.3	-84.3	0.34 (1)	4.69	I- D	-557 / 0	0.14 (1)
D- O	-1683 / 0	-84.3	-84.3	0.32 (1)	4.72	I- E	0 / 578	0.14 (1)
O- E	-1683 / 0	-84.3	-84.3	0.32 (1)	4.72			
E- P	-1270 / 0	-84.3	-84.3	0.29 (1)	5.32			
P- F	-1270 / 0	-84.3	-84.3	0.29 (1)	5.32			
G- F	-1487 / 0	0.0	0.0	0.37 (1)	6.58			
K- B	-1547 / 0	0.0	0.0	0.17 (1)	6.58			
K- O	0 / 0	-28.0	-28.0	0.10 (2)	10.00			
Q- R	0 / 0	-28.0	-28.0	0.10 (2)	10.00			
R- J	0 / 0	-28.0	-28.0	0.10 (2)	10.00			
J- S	0 / 1366	-28.0	-28.0	0.23 (1)	10.00			
S- T	0 / 1366	-28.0	-28.0	0.23 (1)	10.00			
T- I	0 / 1366	-28.0	-28.0	0.23 (1)	10.00			
I- U	0 / 1270	-28.0	-28.0	0.23 (1)	10.00			
U- H	0 / 1270	-28.0	-28.0	0.23 (1)	10.00			
H- V	0 / 0	-28.0	-28.0	0.09 (2)	10.00			
V- G	0 / 0	-28.0	-28.0	0.09 (2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
C	4-0-11	-23	-25	—	FRONT	VERT	DEAD
C	4-0-11	-222	-222	—	FRONT	VERT	SNOW
E	11-6-12	-101	-101	—	BACK	VERT	TOTAL
F	15-9-8	-134	-134	—	BACK	VERT	TOTAL
G	15-9-8	-51	-89	—	BACK	VERT	TOTAL
H	11-6-12	-40	-70	—	BACK	VERT	TOTAL
L	3-6-12	-123	-123	—	BACK	VERT	TOTAL
M	5-6-12	-101	-101	—	BACK	VERT	TOTAL
N	7-6-12	-101	-101	—	BACK	VERT	TOTAL
O	9-6-12	-101	-101	—	BACK	VERT	TOTAL
P	13-6-12	-101	-101	—	BACK	VERT	TOTAL
Q	1-6-12	-40	-70	—	BACK	VERT	TOTAL
R	3-6-12	-40	-70	—	BACK	VERT	TOTAL

[M]

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.53")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.53")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.37/1.00 (B-C:1) , BC=0.23/1.00 (I-J:1) , WB=0.43/1.00 (F-H:1) , BSI=0.29/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (F) (INPUT = 0.90)
JSI METAL= 0.37 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
PROVINCE OF ONTARIO
1318
S. KATSOULAKOS

DWG NO. TAM 5980-8 P6/L

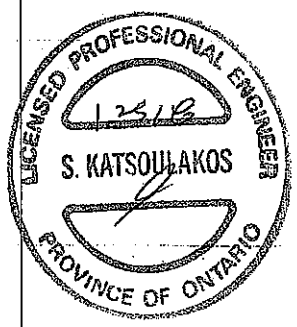
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285864	T30	1	1	TRUSS DESC.	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
S	5-6-12	-40	-70	—	BACK	VERT	TOTAL
T	7-6-12	-40	-70	—	BACK	VERT	TOTAL
U	9-6-12	-40	-70	—	BACK	VERT	TOTAL
V	13-6-12	-40	-70	—	BACK	VERT	TOTAL



DWG NO. TAM 5980-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286043	T31	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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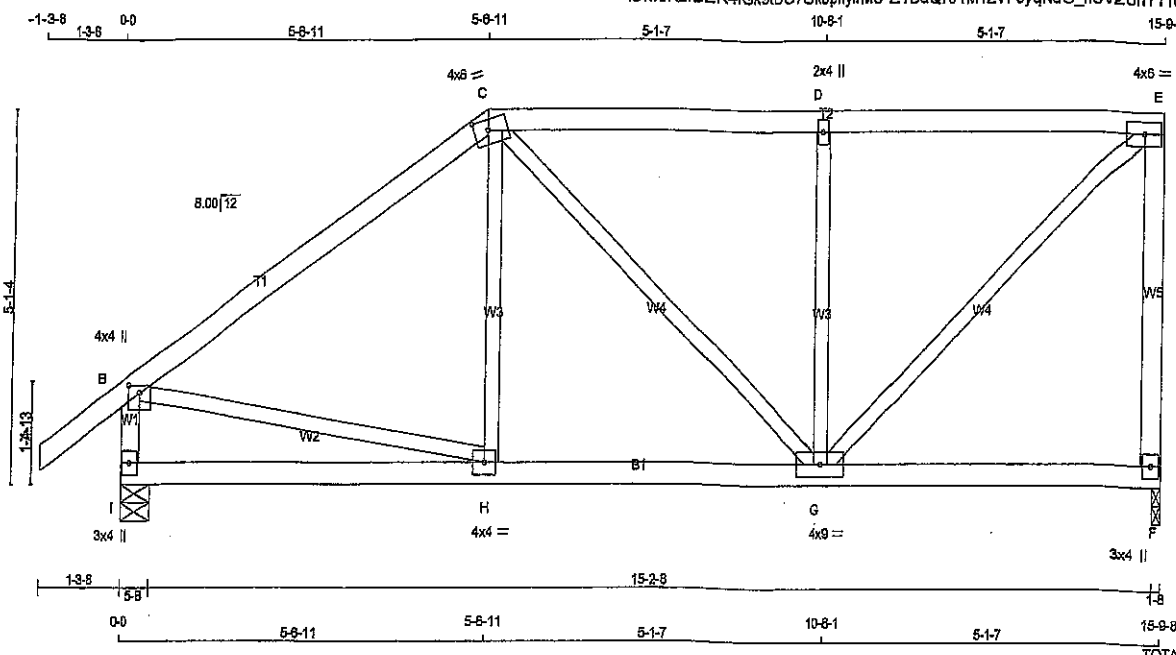

TOTAL WEIGHT = 68 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTWW-m	MT20	4.0	6.0	1.75	2.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	8.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BMWW-t	MT20	4.0	4.0		
I	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	866	0	866	0	0	1-8	1-8
I	1002	0	1002	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	728	404 / 0	168 / 0	0 / 0	0 / 0	156 / 0	0 / 0
I	806	474 / 0	196 / 0	0 / 0	0 / 0	166 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.96 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.3	-84.3	1	0.11 (1)	10.00
B-C	-821 / 0	-84.3	-84.3	1	0.49 (1)	5.96
C-D	-707 / 0	-84.3	-84.3	1	0.36 (1)	6.25
D-E	-707 / 0	-84.3	-84.3	1	0.36 (1)	6.25
F-E	-826 / 0	0.0	0.0	1	0.36 (1)	7.81
I-B	-937 / 0	0.0	0.0	1	0.10 (1)	7.81
I-H	0 / 0	-28.0	-28.0	1	0.19 (3)	10.00
H-G	0 / 683	-28.0	-28.0	1	0.28 (2)	10.00
G-F	0 / 0	-28.0	-28.0	1	0.16 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H-C	0 / 222	0.05 (3)
C-G	-23 / 33	0.02 (3)
G-D	-532 / 0	0.20 (1)
G-E	0 / 967	0.22 (1)
B-H	0 / 895	0.16 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.53")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.53")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.49/1.00 (B-C:1) , BC=0.28/1.00 (G-H:2) , WB=0.22/1.00 (E-G:1) , SSI=0.21/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.60 (B) (INPUT = 0.90)
JSI METAL= 0.28 (H) (INPUT = 1.00)

DWG NO. TAM 3996 -13
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286043	T32	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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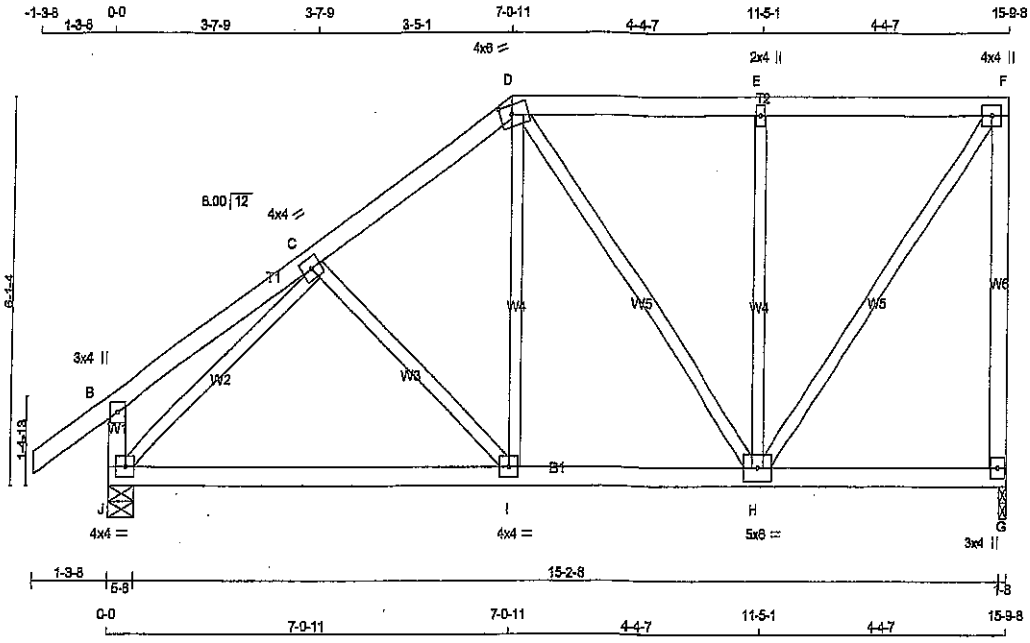


TOTAL WEIGHT = 75 lb [M]

LUMBER
N. L.G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTWW-m	MT20	4.0	6.0	1.75	2.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	4.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	886	0	886	0	0	1-8	1-8
J	1002	0	1002	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SDIL
G	728	404 / 0	168 / 0	0 / 0	0 / 0	156 / 0	0 / 0
J	806	474 / 0	166 / 0	0 / 0	0 / 0	166 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	C- I	-127 / 52	0.05 (1)	
B- C	0 / 18	-84.3	-84.3	0.16 (1)	10.00	I- D	0 / 348	0.08 (2)	
C- D	-756 / 0	-84.3	-84.3	0.17 (1)	6.25	D- H	-168 / 0	0.17 (2)	
D- E	-522 / 0	-84.3	-84.3	0.27 (1)	6.25	H- E	-453 / 0	0.27 (1)	
E- F	-522 / 0	-84.3	-84.3	0.27 (1)	6.25	H- F	0 / 664	0.19 (1)	
G- F	-831 / 0	0.0	0.0	0.60 (1)	7.81	J- C	-987 / 0	0.38 (1)	
J- B	-231 / 0	0.0	0.0	0.02 (1)	7.81				
J- I	0 / 707	-28.0	-28.0	0.38 (2)	10.00				
I- H	0 / 819	-28.0	-28.0	0.38 (2)	10.00				
H- G	0 / 0	-28.0	-28.0	0.13 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.53")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.58")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.60/1.00 (F-G:1) , BC=0.38/1.00 (H-I:2) , WB=0.38/1.00 (C-J:1) , SSI=0.18/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION UVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

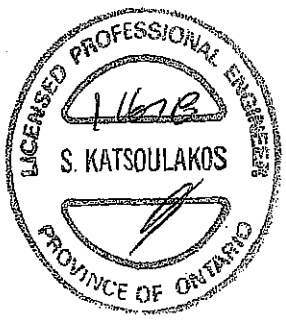
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (F) (INPUT = 0.90)
JSI METAL= 0.35 (C) (INPUT = 1.00)



DWG NO. TAM 3997-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285864	T33Z4	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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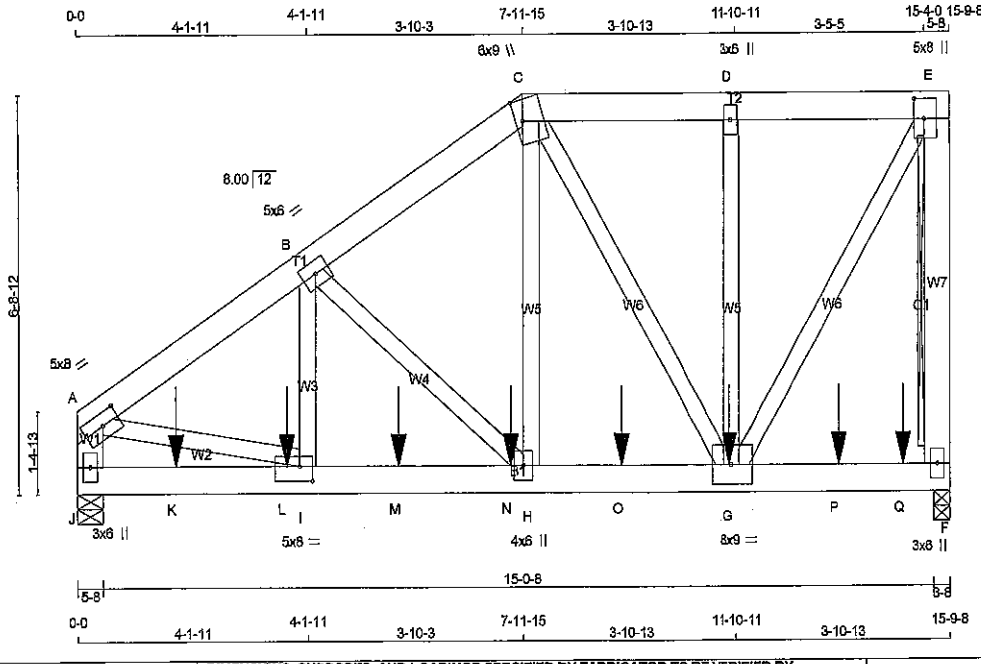


TOTAL WEIGHT = 3 X 114 = 341 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
F - E	2x6	DRY	No.2	SPF
J - A	2x6	DRY	No.2	SPF
J - F	2x8	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS		SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	2	12	TOP
C-E	2	12	TOP
E-F	2	12	TOP
J-A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
J-F	2	7	SIDE(893.7)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	8.0	2.50	3.75
B	TMWW-t	MT20	5.0	6.0		
C	TTWW+m	MT20	6.0	9.0	4.25	1.50
D	TMW+w	MT20	3.0	6.0		
E	TMVW+p	MT20	5.0	8.0	4.00	2.25
F	BMV1+p	MT20	3.0	6.0		
G	BMWWW-t	MT20	8.0	9.0		
H	BMWW+t	MT20	4.0	6.0		
I	BMWW-t	MT20	5.0	8.0	3.00	2.75
J	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES
1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	8425	0	8425	0	0	3-8	3-8
J	7250	0	7250	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEA0	SOIL
F	6663	3898 / 0	1505 / 0	0 / 0	0 / 0	1460 / 0	0 / 0
J	5882	3379 / 0	1264 / 0	0 / 0	0 / 0	1239 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.97 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x6 DRY SPF No.2 T-BRACE AT E-F

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	-8297 / 0	-84.3	-84.3	0.09 (1)	4.97	I-B	0 / 2886	0.14 (1)
B-C	-6004 / 0	-84.3	-84.3	0.07 (1)	5.85	B-H	-2573 / 0	0.25 (1)
C-D	-4034 / 0	-84.3	-84.3	0.05 (1)	6.25	H-C	0 / 4732	0.25 (1)
D-E	-4034 / 0	-84.3	-84.3	0.04 (1)	6.25	C-G	-2055 / 0	0.45 (1)
F-E	-6509 / 0	0.0	0.0	0.26 (1)	7.81	G-D	-276 / 20	0.04 (1)
J-A	-8286 / 0	0.0	0.0	0.13 (1)	7.01	G-E	0 / 7355	0.39 (1)
						A-I	0 / 7052	0.36 (1)
J-K	0 / 0	-28.0	-28.0	0.20 (1)	10.00			
K-L	0 / 0	-28.0	-28.0	0.20 (1)	10.00			
L-I	0 / 0	-28.0	-28.0	0.20 (1)	10.00			
I-M	0 / 6892	-28.0	-28.0	0.26 (1)	10.00			
M-N	0 / 6892	-28.0	-28.0	0.26 (1)	10.00			
N-H	0 / 6892	-28.0	-28.0	0.26 (1)	10.00			
H-O	0 / 5105	-28.0	-28.0	0.24 (1)	10.00			
O-G	0 / 5105	-28.0	-28.0	0.24 (1)	10.00			
G-P	0 / 302	-28.0	-28.0	0.23 (1)	10.00			
P-Q	0 / 302	-28.0	-28.0	0.23 (1)	10.00			
Q-F	0 / 302	-28.0	-28.0	0.23 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
G	11-9-4	-1712	-1712	—	FRONT	VERT	TOTAL
K	1-9-4	-1826	-1826	—	FRONT	VERT	TOTAL
L	3-9-4	-1826	-1826	—	FRONT	VERT	TOTAL
M	5-9-4	-1712	-1712	—	FRONT	VERT	TOTAL
N	7-9-4	-1712	-1712	—	FRONT	VERT	TOTAL
O	9-9-4	-1712	-1712	—	FRONT	VERT	TOTAL
P	13-9-4	-1712	-1712	—	FRONT	VERT	TOTAL
Q	14-11-4	-1714	-1714	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.53")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.53")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.26/1.00 (E-F:1) , BC=0.26/1.00 (H-I:1) , WB=0.46/1.00 (C-G:1) , SSI=0.56/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

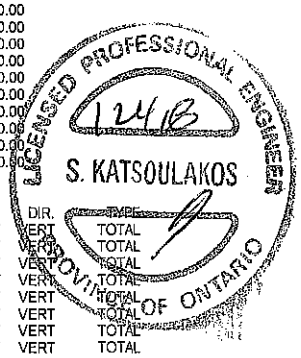
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.52 (A) (INPUT = 1.00)



PG 1/2

DWG NO. TAM 5091-18
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285864	T33Z4	1	3	TRUSS DESC.	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1826.2 lbs FACTORED DOWN AT 1-9-4, 1826.2 lbs FACTORED DOWN AT 3-9-4, 1711.9 lbs FACTORED DOWN AT 5-9-4, 1711.9 lbs FACTORED DOWN AT 7-9-4, 1711.9 lbs FACTORED DOWN AT 9-9-4, 1711.9 lbs FACTORED DOWN AT 11-9-4, AND 1711.9 lbs FACTORED DOWN AT 13-9-4, AND 1714.0 lbs FACTORED DOWN AT 14-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
124B
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 5091 -R
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
286039	T37	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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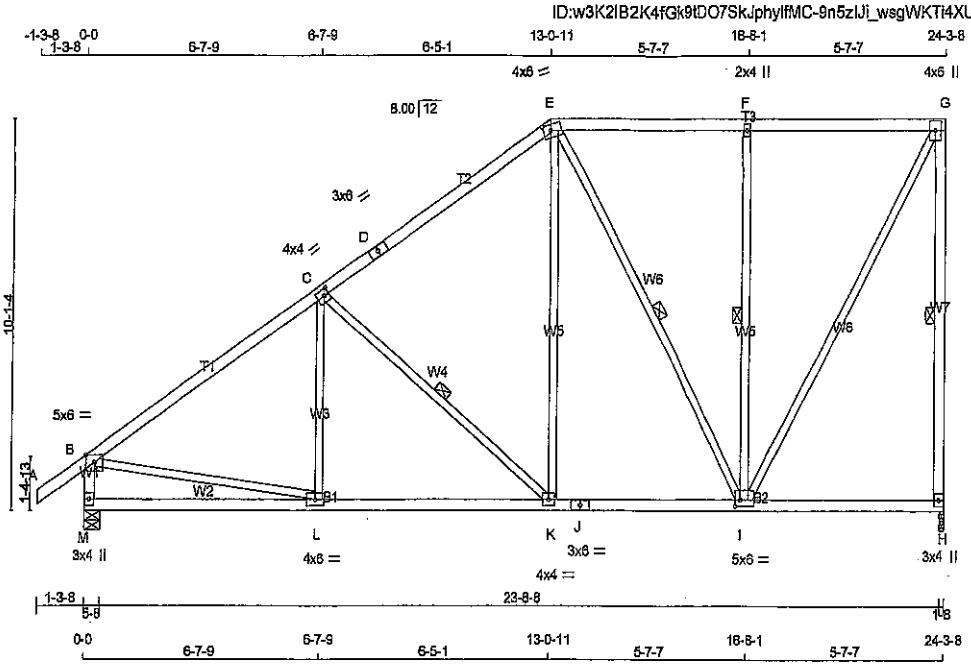


TOTAL WEIGHT = 2 X 122 = 243 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	4.0	6.0	1.75	2.50
F	TMW+w	MT20	2.0	4.0		
G	TMVW+p	MT20	4.0	8.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWWW-t	MT20	5.0	6.0	2.00	1.50
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H	1363	0	1363	0	0	1-8	1-8
M	1479	0	1479	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1119	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
M	1196	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.69 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

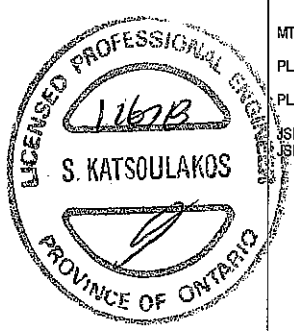
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-H, C-K, E-I, F-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.3	-84.3	1	0.11 (1)	10.00
B-C	-1522 / 0	-84.3	-84.3	1	0.55 (1)	4.89
C-D	-1051 / 0	-84.3	-84.3	1	0.49 (1)	5.46
D-E	-1051 / 0	-84.3	-84.3	1	0.49 (1)	5.46
E-F	-644 / 0	-84.3	-84.3	1	0.34 (1)	6.25
F-G	-644 / 0	-84.3	-84.3	1	0.34 (1)	6.25
H-G	-1299 / 0	0.0	0.0	1	0.65 (1)	5.66
M-B	-1404 / 0	0.0	0.0	1	0.14 (1)	6.87
M-L	0 / 0	-28.0	-28.0	1	0.31 (3)	10.00
L-K	0 / 1298	-28.0	-28.0	1	0.46 (2)	10.00
K-J	0 / 848	-28.0	-28.0	1	0.31 (2)	10.00
J-I	0 / 848	-28.0	-28.0	1	0.31 (2)	10.00
I-H	0 / 0	-28.0	-28.0	1	0.21 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L-C	-1 / 267	0.06 (3)
C-K	-594 / 0	0.30 (1)
K-E	0 / 545	0.12 (2)
E-I	-413 / 0	0.40 (1)
I-F	-566 / 0	0.41 (1)
I-G	0 / 1269	0.29 (1)
B-L	0 / 1314	0.30 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.65/1.00 (G-H:1) , BC=0.46/1.00 (K-L:2) , WB=0.41/1.00 (F-I:1) , SSI=0.23/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SSI GRIP= 0.83 (L) (INPUT = 0.90)
SSI METAL= 0.32 (L) (INPUT = 1.00)

DWG NO. TAM3994-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285876	T73S	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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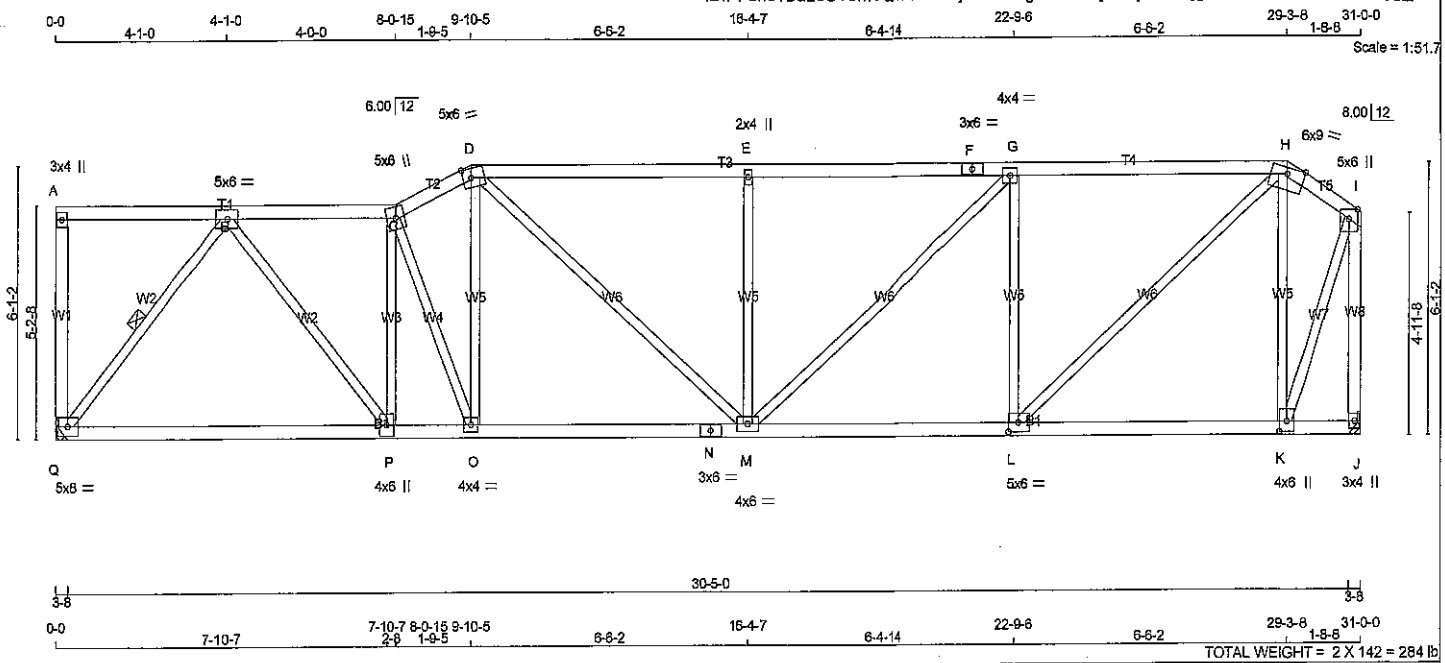

TOTAL WEIGHT = 2 X 142 = 284 lb

(M)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
Q - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - I	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	5.0	6.0		
C	TTWW+m	MT20	5.0	6.0		
D	TTWW-m	MT20	5.0	6.0	2.50	2.25
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW-m	MT20	6.0	9.0	Edge	
I	TMVW+p	MT20	5.0	6.0	Edge	
J	BMV1+p	MT20	3.0	4.0		
K	BMWW+t	MT20	4.0	6.0	2.75	2.00
L	BMWW-t	MT20	5.0	6.0	2.50	2.75
M	BMWWW-t	MT20	4.0	6.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWW+t	MT20	4.0	6.0		
Q	BMVW1-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
Q	1740	0	1740	0	0	IN-SX	IN-SX
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-8	
J	1740	0	1740	0	0	HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1429	793 / 0	326 / 0	0 / 0	0 / 0	310 / 0	0 / 0
J	1429	793 / 0	326 / 0	0 / 0	0 / 0	310 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.74 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF B-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
Q- A	-131 / 0	0.0	0.0	0.06 (1)	7.81
A- B	0 / 0	-84.3	-84.3	0.23 (1)	10.00
B- C	-2131 / 0	-84.3	-84.3	0.30 (1)	4.36
C- D	-2260 / 0	-84.3	-84.3	0.10 (1)	4.48
D- E	-2390 / 0	-84.3	-84.3	0.69 (1)	3.75
E- F	-2391 / 0	-84.3	-84.3	0.69 (1)	3.74
F- G	-2391 / 0	-84.3	-84.3	0.69 (1)	3.74
G- H	-1885 / 0	-84.3	-84.3	0.62 (1)	4.19
H- I	-628 / 0	-84.3	-84.3	0.05 (1)	6.25
J- I	-1756 / 0	0.0	0.0	0.77 (1)	6.29
Q- P	0 / 1260	-28.0	-28.0	0.54 (2)	10.00
P- O	0 / 2150	-28.0	-28.0	0.66 (2)	10.00
O- N	0 / 2028	-28.0	-28.0	0.48 (2)	10.00
N- M	0 / 2028	-28.0	-28.0	0.48 (2)	10.00
M- L	0 / 1885	-28.0	-28.0	0.46 (2)	10.00
L- K	0 / 500	-28.0	-28.0	0.29 (2)	10.00
K- J	0 / 0	-28.0	-28.0	0.18 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- C	-882 / 0	0.35 (1)
C- O	-359 / 0	0.18 (1)
O- D	0 / 399	0.09 (2)
K- H	-1178 / 0	0.69 (1)
K- I	0 / 1430	0.32 (1)
L- H	0 / 1854	0.42 (1)
D- M	0 / 485	0.11 (1)
L- G	-1051 / 0	0.61 (1)
M- E	-592 / 0	0.35 (1)
M- G	0 / 682	0.15 (1)
Q- B	-1972 / 0	0.55 (1)
B- P	0 / 1402	0.32 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.30")

CSI: TC=0.77 (I-J:1) , BC=0.66 (O-P:2) , WB=0.69 (H-K:1) , SSI=0.26 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1867	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (D) (INPUT = 0.90)
JSI METAL= 0.55 (N) (INPUT = 1.00)

DWG NO. T4M45036-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285876	T74S	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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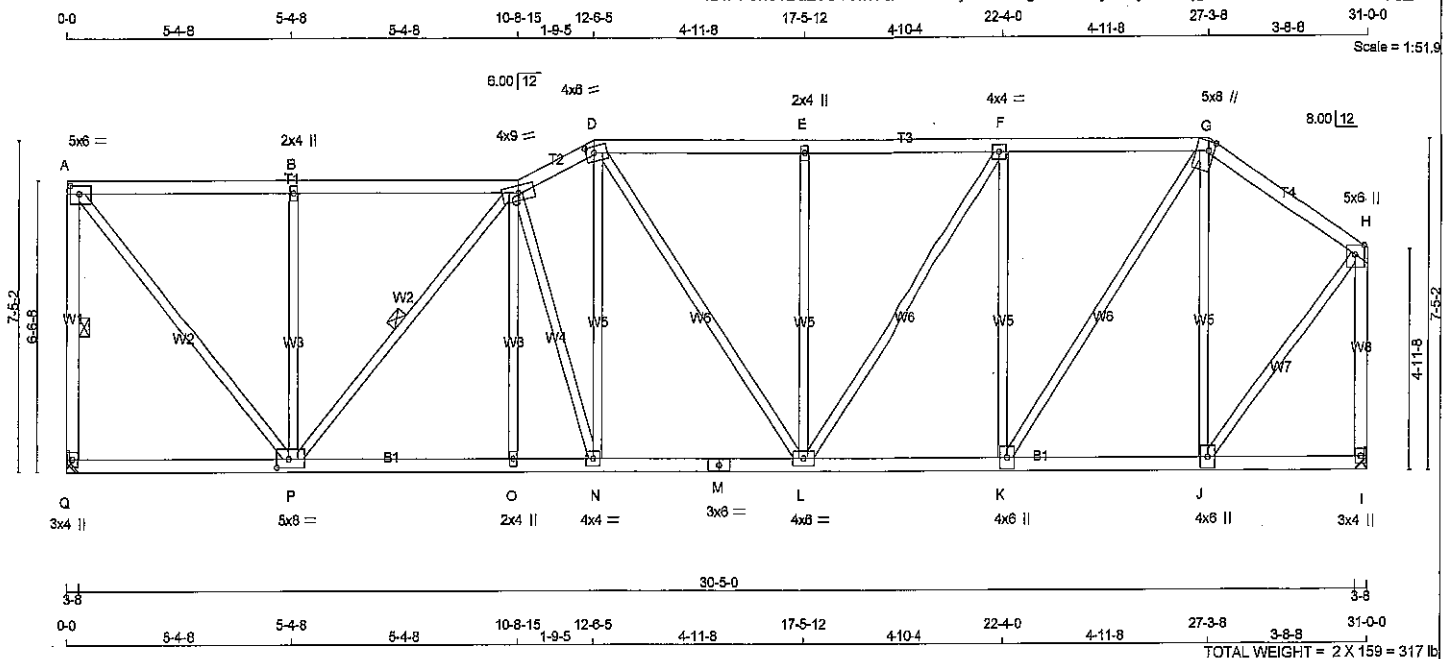

TOTAL WEIGHT = 2 X 159 = 317 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
Q - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0	2.25	2.50
B	TMW+w	MT20	2.0	4.0		
C	TTWWW-m	MT20	4.0	9.0		
D	TTWW-m	MT20	4.0	6.0	1.75	2.25
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTWW+m	MT20	5.0	8.0	2.50	1.50
H	TMVW+p	MT20	5.0	6.0	Edge	
I	BMV1+p	MT20	3.0	4.0		
J	BMWW+t	MT20	4.0	6.0		
K	BMWW+t	MT20	4.0	6.0		
L	BMWWW-t	MT20	4.0	6.0		
M	BS-t	MT20	3.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMW+w	MT20	2.0	4.0		
P	BMWWW-t	MT20	5.0	8.0	2.25	3.50
Q	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
Q	1740	0	1740	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	
I	1740	0	1740	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1429	793 / 0	326 / 0	0 / 0	0 / 0	310 / 0	0 / 0
I	1429	793 / 0	326 / 0	0 / 0	0 / 0	310 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.53 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF A-Q, C-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	CSI (LC)	MAX UNBRAC LENGTH
Q-A	-1679 / 0	0.0	0.0	0.33 (1)	5.13
A-B	-1304 / 0	-84.3	-84.3	0.45 (1)	5.06
B-C	-1304 / 0	-84.3	-84.3	0.45 (1)	5.06
C-D	-2035 / 0	-84.3	-84.3	0.09 (1)	4.69
D-E	-1892 / 0	-84.3	-84.3	0.33 (1)	4.55
E-F	-1892 / 0	-84.3	-84.3	0.34 (1)	4.53
F-G	-1556 / 0	-84.3	-84.3	0.32 (1)	4.93
G-H	-986 / 0	-84.3	-84.3	0.22 (1)	6.01
I-H	-1702 / 0	0.0	0.0	0.75 (1)	6.37
Q-P	0 / 0	-28.0	-28.0	0.21 (3)	10.00
P-O	0 / 1961	-28.0	-28.0	0.44 (2)	10.00
O-N	0 / 1980	-28.0	-28.0	0.37 (1)	10.00
N-M	0 / 1829	-28.0	-28.0	0.36 (1)	10.00
M-L	0 / 1829	-28.0	-28.0	0.36 (1)	10.00
L-K	0 / 1556	-28.0	-28.0	0.32 (1)	10.00
K-J	0 / 807	-28.0	-28.0	0.23 (2)	10.00
J-I	0 / 0	-28.0	-28.0	0.13 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-P	0 / 1991	0.45 (1)
P-B	-559 / 0	0.39 (1)
P-C	-1011 / 0	0.45 (1)
O-C	0 / 148	0.03 (3)
C-N	-484 / 0	0.37 (1)
N-D	0 / 570	0.13 (1)
J-G	-858 / 0	0.85 (1)
J-H	0 / 1290	0.29 (1)
K-G	0 / 1312	0.30 (1)
D-L	0 / 111	0.03 (1)
K-F	-943 / 0	0.93 (1)
L-E	-448 / 0	0.44 (1)
L-F	0 / 598	0.13 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.75 (H-I:1) , BC=0.44 (O-P:2) , WB=0.93 (F-K:1) , SSI=0.22 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (P) (INPUT = 0.90)
JSI METAL= 0.62 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM45037-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43854	DRWG NO.
285876	T90TCX	18	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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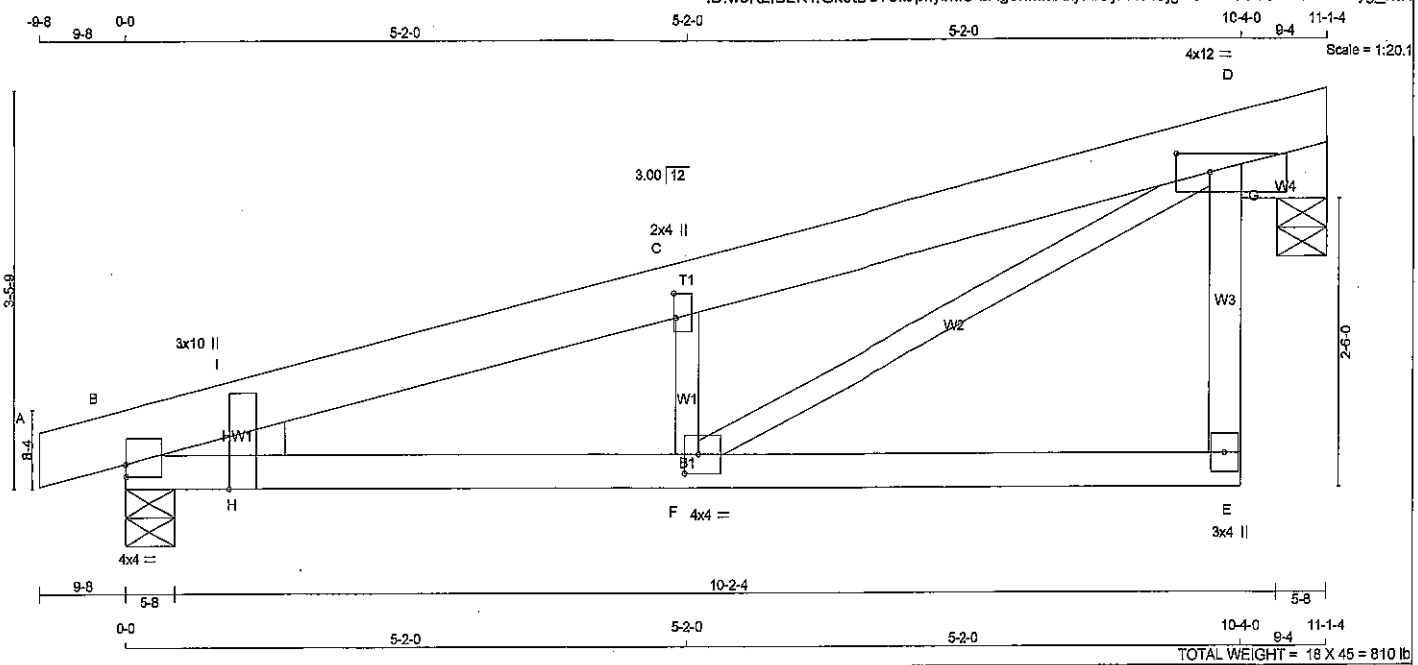

TOTAL WEIGHT = 18 X 45 = 810 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	4.0	4.0	1.25	
B	WP+I	MT20	3.0	10.0	Edge	11.50
C	TMW+w	MT20	2.0	4.0	2.50	0.25
D	TMVWW1-p	MT20	4.0	12.0	2.00	3.75
E	BMV+p	MT20	3.0	4.0		
F	BMWW-t	MT20	4.0	4.0	2.00	1.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
J(D)	551	0	551	0	0	5-8	5-8	
B	660	0	660	0	0	5-8	5-8	2x4 L

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J(D)	452	251 / 0	103 / 0	0 / 0	0 / 0	98 / 0	0 / 0
B	548	321 / 0	114 / 0	0 / 0	0 / 0	114 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J(D), B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-4 / 0	-84.3	-84.3	0.02 (1)	10.00
B- I	-1258 / 0	-84.3	-84.3	0.03 (1)	6.25
I- C	-1158 / 0	-84.3	-84.3	0.07 (1)	6.25
C- D	-1152 / 0	-84.3	-84.3	0.17 (1)	6.25
E- G	0 / 106	0.0	0.0	0.34 (1)	10.00
G- D	0 / 106	0.0	0.0	0.34 (1)	10.00
B- H	0 / 1128	-28.0	-28.0	0.20 (1)	10.00
H- F	0 / 1128	-28.0	-28.0	0.28 (2)	10.00
F- E	0 / 103	-28.0	-28.0	0.17 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	-420 / 0	0.06 (1)
F- D	0 / 1157	0.26 (1)
H- I	0 / 131	0.00 (1)
D- J	-785 / 0	0.03 (1)
G- J	0 / 558	0.00 (1)

DWG NO. TAM 45038-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.34 (E-G:1) , BC=0.28 (F-H:2) , WB=0.26 (D-F:1) , SSI=0.15 (D-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (F) (INPUT = 0.90)
JSI METAL= 0.40 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285876	G90	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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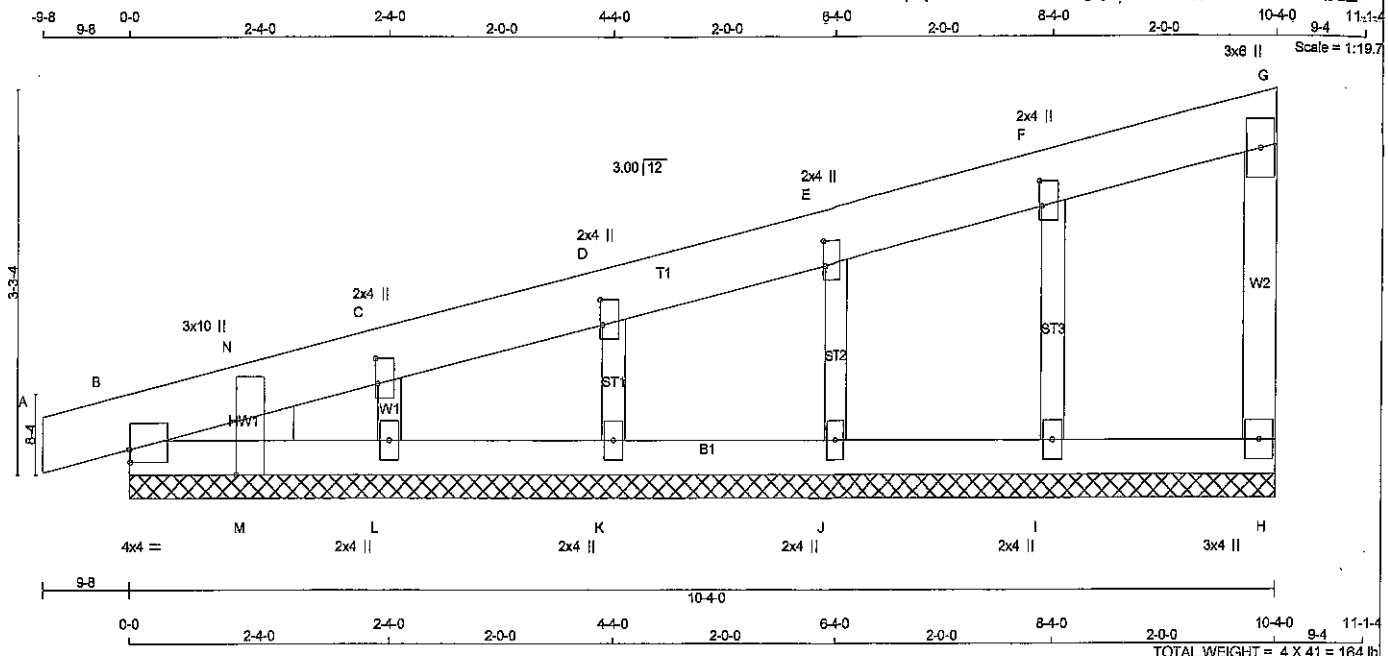

TOTAL WEIGHT = 4 X 41 = 164 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - G	2x6	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
B - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	4.0	4.0	1.25	
B	WP+I	MT20	3.0	10.0	Edge	11.50
C, D, E, F						
C	TMW+w	MT20	2.0	4.0	2.50	0.25
G	TMV+p	MT20	3.0	8.0		
H	BMV1+p	MT20	3.0	4.0		
I, J, K, L						
I	BMW1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

HEEL
WEDGE
2x4 L
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-4 / 0	-84.3	-84.3	0.02 (1)	10.00	I- F	-186 / 0	0.03 (1)
B- N	-48 / 0	-84.3	-84.3	0.02 (1)	6.25	J- E	-168 / 0	0.03 (1)
N- C	-14 / 3	-84.3	-84.3	0.03 (1)	6.25	K- D	-158 / 0	0.02 (1)
C- D	-18 / 0	-84.3	-84.3	0.03 (1)	6.25	L- C	-201 / 0	0.03 (1)
D- E	-9 / 0	-84.3	-84.3	0.02 (1)	10.00	M- N	0 / 41	0.00 (1)
E- F	-4 / 0	-84.3	-84.3	0.02 (1)	10.00			
F- G	-4 / 0	-84.3	-84.3	0.02 (1)	10.00			
H- G	-72 / 0	0.0	0.0	0.01 (1)	7.81			
B- M	0 / 28	-28.0	-28.0	0.02 (1)	10.00			
M- L	0 / 28	-28.0	-28.0	0.03 (2)	10.00			
L- K	0 / 16	-28.0	-28.0	0.02 (2)	10.00			
K- J	0 / 9	-28.0	-28.0	0.02 (2)	10.00			
J- I	0 / 4	-28.0	-28.0	0.03 (2)	10.00			
I- H	0 / 0	-28.0	-28.0	0.03 (3)	10.00			

DWG NO. TAM 45067-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.03 (C-D:1) , BC=0.03 (I-J:2) , WB=0.03 (F-I:1) , SSI=0.07 (C-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (C) (INPUT = 0.90)
JSI METAL= 0.04 (L) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285862	T91	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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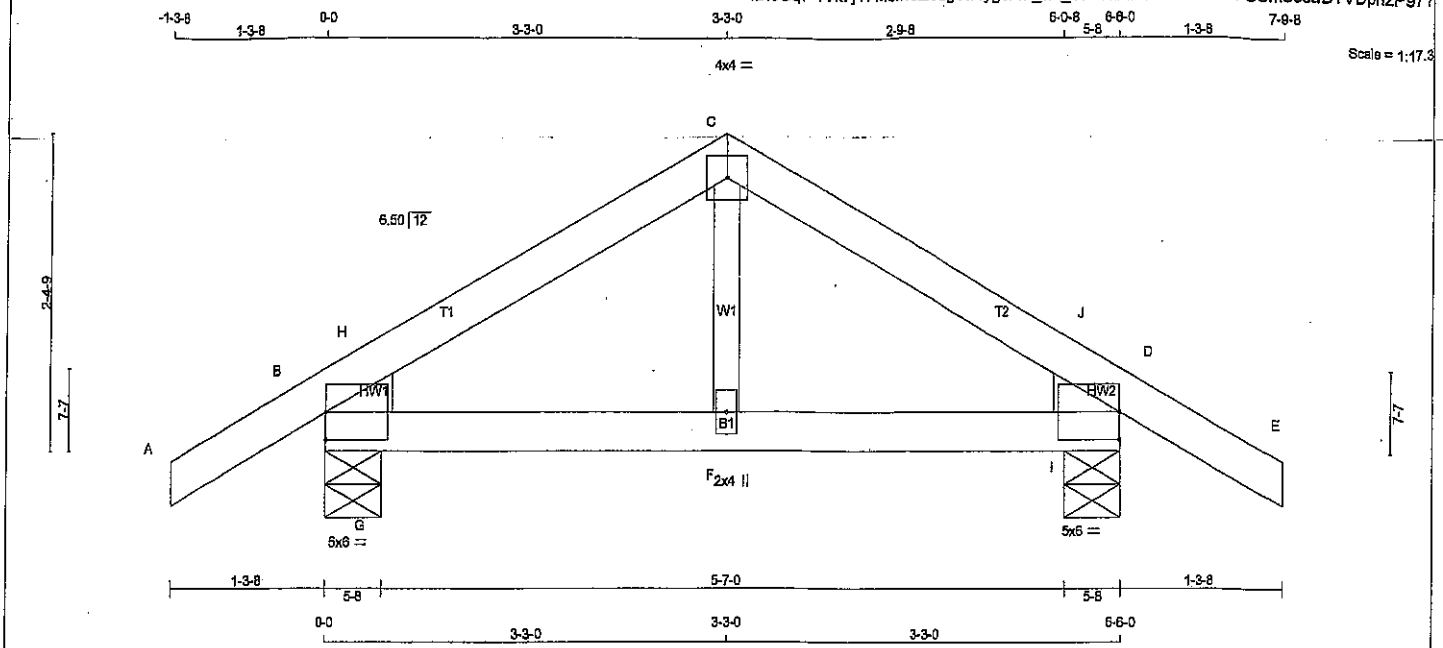

TOTAL WEIGHT = 3 X 22 = 67 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-I	MT20	5.0	6.0	2.50	
C	TTW-p	MT20	4.0	4.0		
D	TMBH1-I	MT20	5.0	6.0	2.50	
F	BMW+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
B	480	0	480	0	0	5-8	5-8	2x4 L
D	480	0	480	0	0	5-8	5-8	2x4 R

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	377	236 / 0	68 / 0	0 / 0	0 / 0	73 / 0	0 / 0
D	377	236 / 0	68 / 0	0 / 0	0 / 0	73 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 17	-84.3	-84.3	0.11 (1)		10.00	F- C	0 / 165	0.04 (2)
B- H	-411 / 0	-84.3	-84.3	0.05 (1)		6.25	G- H	-34 / 113	0.00 (1)
H- C	-355 / 0	-84.3	-84.3	0.09 (1)		6.25	I- J	-33 / 114	0.00 (1)
C- J	-355 / 0	-84.3	-84.3	0.08 (1)		6.25			
J- D	-412 / 0	-84.3	-84.3	0.05 (1)		6.25			
D- E	0 / 17	-84.3	-84.3	0.11 (1)		10.00			
B- G	0 / 305	-28.0	-28.0	0.08 (1)		10.00			
G- F	0 / 305	-28.0	-28.0	0.10 (1)		10.00			
F- I	0 / 305	-28.0	-28.0	0.10 (1)		10.00			
I- D	0 / 305	-28.0	-28.0	0.08 (1)		10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.11/1.00 (A-B:1) , BC=0.10/1.00 (F-G:1) , WB=0.04/1.00 (C-F:2) , SSI=0.09/1.00 (C-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2264	1655

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (D) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/19/18
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 21210-RB
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285862	G91	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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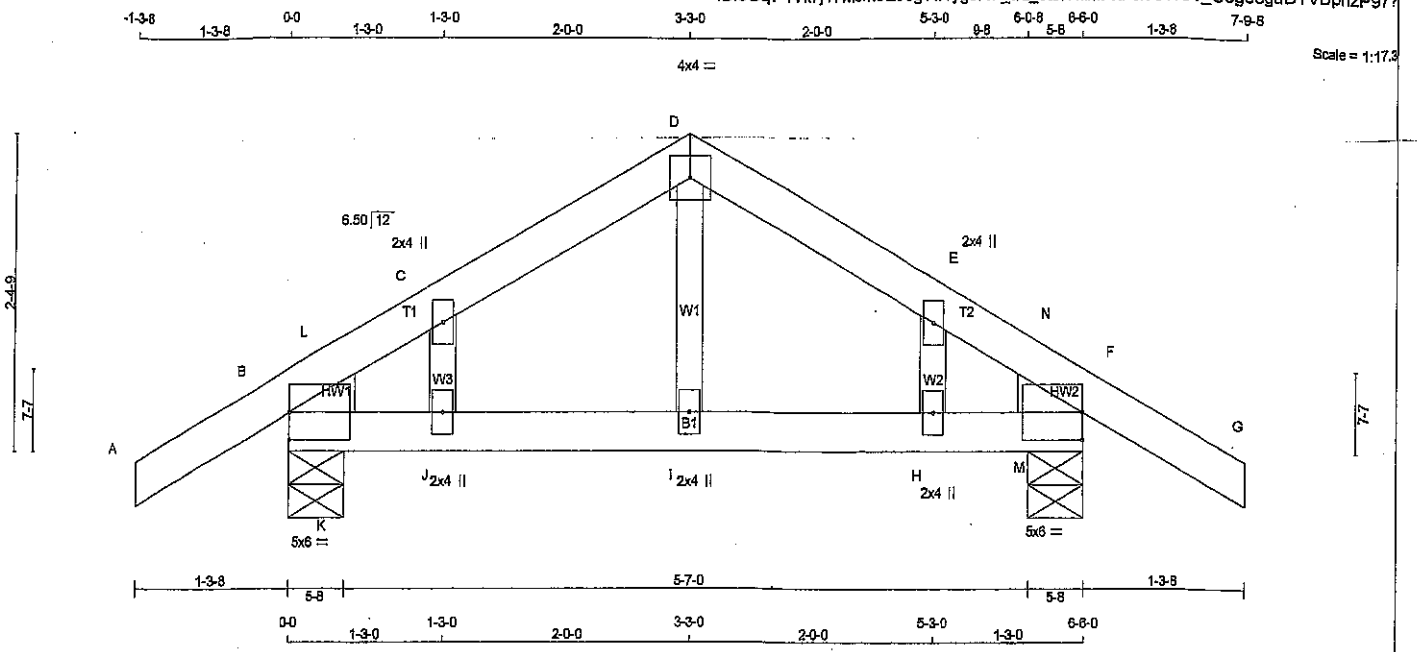

TOTAL WEIGHT = 23 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-I	MT20	5.0	6.0	2.50	
C	TMW+w	MT20	2.0	4.0		
D	TTW-p	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TMBH1-I	MT20	5.0	6.0	2.50	
H, I, J						
H	BMW+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
B	480	0	480	0	0	5-8	5-8	2x4 L
F	480	0	480	0	0	5-8	5-8	2x4 R

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	377	236 / 0	68 / 0	0 / 0	0 / 0	73 / 0	0 / 0
F	377	236 / 0	68 / 0	0 / 0	0 / 0	73 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 17	-84.3	-84.3	0.11 (1)	10.00	I- D	0 / 151	0.03 (2)
B- L	-525 / 0	-84.3	-84.3	0.11 (1)	6.25	H- E	-81 / 26	0.01 (1)
L- C	-322 / 0	-84.3	-84.3	0.05 (1)	6.25	J- C	-81 / 26	0.01 (1)
C- D	-298 / 0	-84.3	-84.3	0.06 (1)	6.25	K- L	0 / 225	0.00 (1)
D- E	-298 / 0	-84.3	-84.3	0.06 (1)	6.25	M- N	0 / 227	0.00 (1)
E- N	-322 / 0	-84.3	-84.3	0.05 (1)	6.25			
N- F	-527 / 0	-84.3	-84.3	0.11 (1)	6.25			
F- G	0 / 17	-84.3	-84.3	0.11 (1)	10.00			
B- K	0 / 267	-28.0	-28.0	0.11 (1)	10.00			
K- J	0 / 267	-28.0	-28.0	0.11 (1)	10.00			
J- I	0 / 261	-28.0	-28.0	0.08 (1)	10.00			
I- H	0 / 261	-28.0	-28.0	0.08 (1)	10.00			
H- M	0 / 267	-28.0	-28.0	0.11 (1)	10.00			
M- F	0 / 267	-28.0	-28.0	0.11 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.8	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.11/1.00 (F-N:1) , BC=0.11/1.00 (H-M:1) , WB=0.03/1.00 (D-I:2) , SSI=0.10/1.00 (F-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (F) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)

DWG NO. TAM 042098
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285876	T92	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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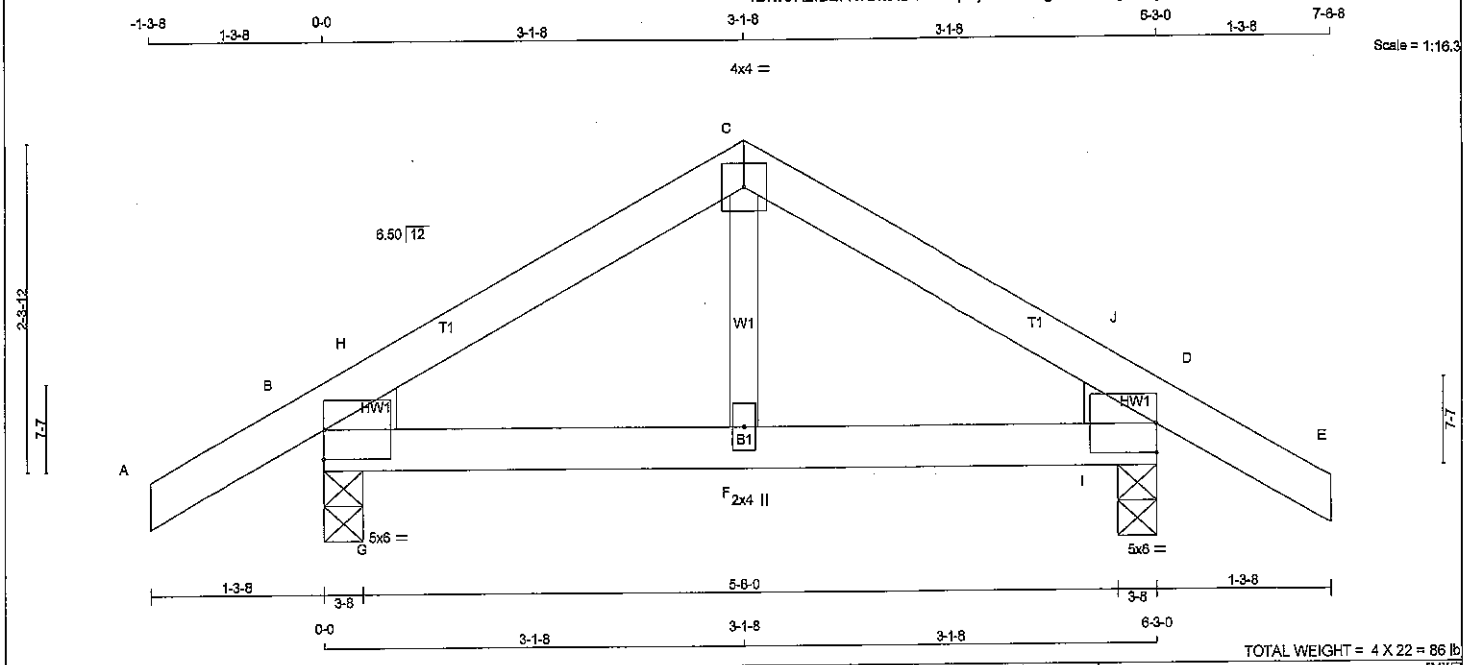

TOTAL WEIGHT = 4 X 22 = 86 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMBH1-I	MT20	5.0	6.0	
C	TTW-p	MT20	4.0	4.0	
D	TMBH1-I	MT20	5.0	6.0	
F	BMW+w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
B	465	0	465	0	0	3-8	3-8	2x4 L
D	465	0	465	0	0	3-8	3-8	2x4 R

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	366	230 / 0	66 / 0	0 / 0	0 / 0	71 / 0	0 / 0
D	366	230 / 0	66 / 0	0 / 0	0 / 0	71 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 17	-84.3	-84.3	0.11 (1)	10.00	
B- H	-397 / 0	-84.3	-84.3	0.04 (1)	6.25	
H- C	-338 / 0	-84.3	-84.3	0.07 (1)	6.25	
C- J	-338 / 0	-84.3	-84.3	0.07 (1)	6.25	
J- D	-397 / 0	-84.3	-84.3	0.04 (1)	6.25	
D- E	0 / 17	-84.3	-84.3	0.11 (1)	10.00	
B- G	0 / 291	-28.0	-28.0	0.08 (1)	10.00	
G- F	0 / 291	-28.0	-28.0	0.09 (1)	10.00	
F- I	0 / 291	-28.0	-28.0	0.09 (1)	10.00	
I- D	0 / 291	-28.0	-28.0	0.08 (1)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	0 / 157	0.04 (2)
G- H	-28 / 107	0.00 (1)
I- J	-28 / 107	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.21")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.21")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.11 (D-E:1) , BC=0.09 (F-G:1) , WB=0.04 (C-F:2) , SSI=0.09 (C-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	822	2284	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (B) (INPUT = 0.90)
JSI METAL= 0.10 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 45042-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285876	T92Z4	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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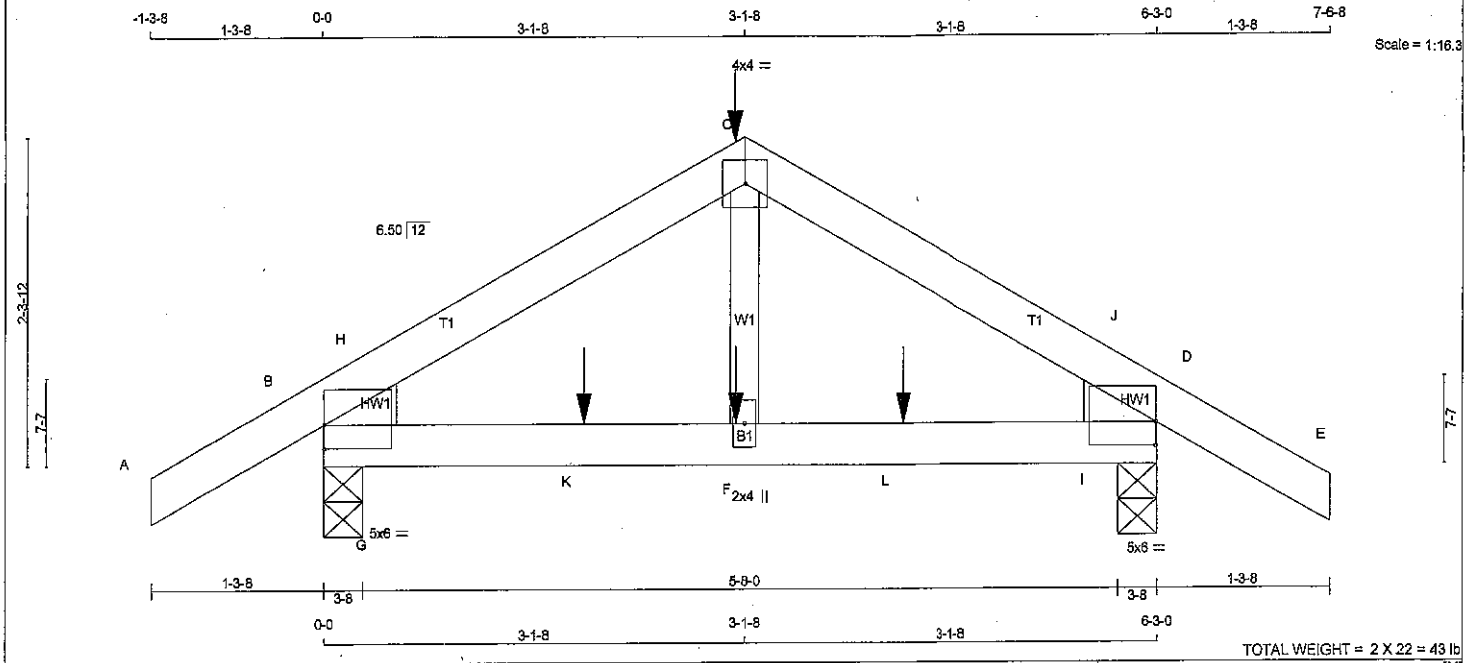

TOTAL WEIGHT = 2 X 22 = 43 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-I	MT20	5.0	6.0	2.00	
C	TTW-p	MT20	4.0	4.0		
D	TMBH1-I	MT20	5.0	6.0	2.00	
F	BMW+w	MT20	2.0	4.0		

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 389.8 lbs FACTORED DOWN AT 3-1-8 ON TOP CHORD, AND 11.9 lbs FACTORED DOWN AND 2.9 lbs FACTORED UP AT 1-11-4, AND 31.5 lbs FACTORED DOWN AT 3-0-12, AND 11.9 lbs FACTORED DOWN AND 2.9 lbs FACTORED UP AT 4-3-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
B	673	0	673	0	0	3-8	3-8	2x4 L
D	673	0	673	0	0	3-8	3-8	2x4 R

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	525	339 / 0	90 / 0	0 / 0	0 / 0	96 / 0	0 / 0
D	525	339 / 0	90 / 0	0 / 0	0 / 0	96 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 17	-84.3	-84.3	0.12 (1)	10.00	F-C	0 / 162	0.04 (3)
B-H	-839 / 0	-84.3	-84.3	0.11 (1)	6.25	G-H	0 / 169	0.00 (1)
H-C	-689 / 0	-84.3	-84.3	0.11 (1)	6.25	I-J	0 / 169	0.00 (1)
C-J	-689 / 0	-84.3	-84.3	0.11 (1)	6.25			
J-D	-839 / 0	-84.3	-84.3	0.11 (1)	8.25			
D-E	0 / 17	-84.3	-84.3	0.12 (1)	10.00			
B-G	0 / 595	-28.0	-28.0	0.15 (1)	10.00			
G-K	0 / 595	-28.0	-28.0	0.15 (1)	10.00			
K-F	0 / 595	-28.0	-28.0	0.15 (1)	10.00			
F-L	0 / 595	-28.0	-28.0	0.15 (1)	10.00			
L-I	0 / 595	-28.0	-28.0	0.15 (1)	10.00			
I-D	0 / 595	-28.0	-28.0	0.15 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	3-1-8	-390	-390	—	FRONT	VERT	TOTAL
F	3-0-12	-31	-31	—	FRONT	VERT	TOTAL
K	1-11-4	3	-12	3	FRONT	VERT	TOTAL
L	4-3-12	3	-12	3	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.21")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.21")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.12 (D-E:1) , BC=0.15 (D-I:1) , WB=0.04 (C-F:3) , SSI=0.11 (C-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (D) (INPUT = 0.90)
JSI METAL= 0.15 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM45041-17
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285876	T93	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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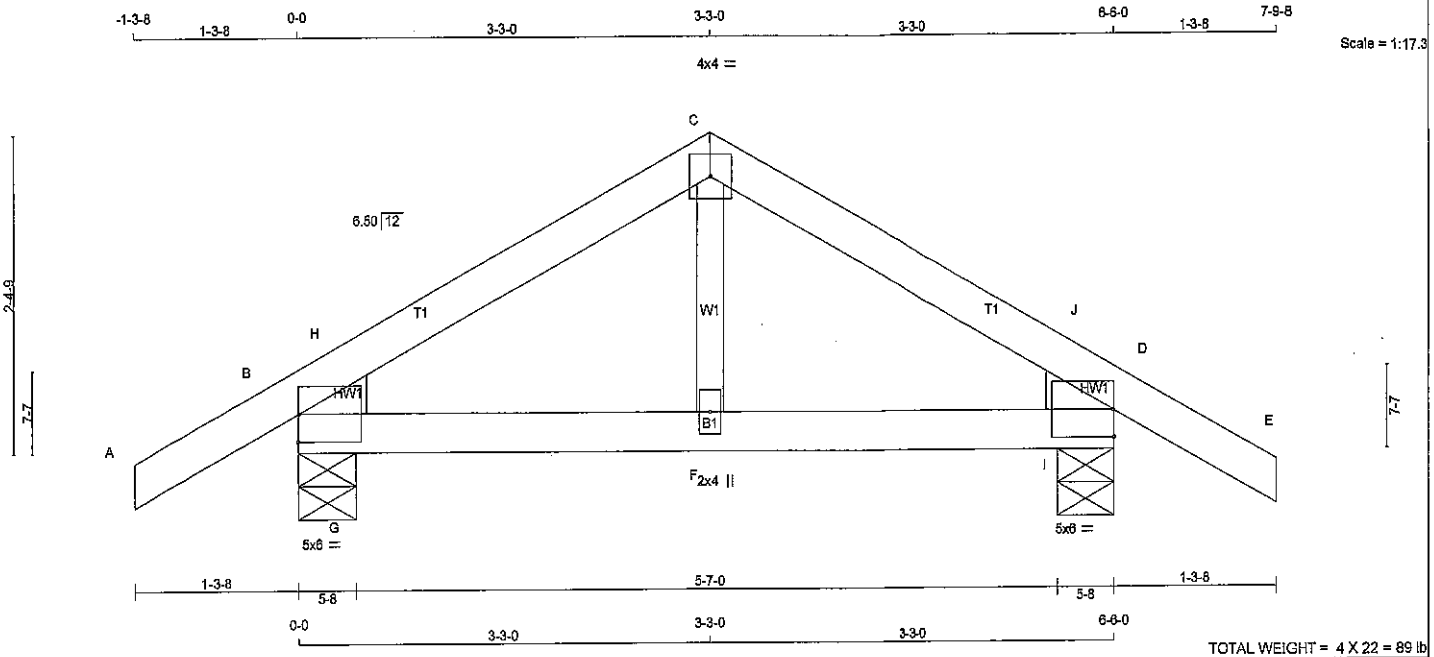

TOTAL WEIGHT = 4 X 22 = 89 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-I	MT20	5.0	6.0		
C	TTW-p	MT20	4.0	4.0		
D	TMBH1-I	MT20	5.0	6.0		
F	BMW+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
B	480	0	480	0	0	5-8	5-8	2x4 L
D	480	0	480	0	0	5-8	5-8	2x4 R

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	377	236 / 0	68 / 0	0 / 0	0 / 0	73 / 0	0 / 0
D	377	236 / 0	68 / 0	0 / 0	0 / 0	73 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF) CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 17	-84.3	-84.3	0.11 (1)	10.00
B-H	-412 / 0	-84.3	-84.3	0.05 (1)	6.25
H-C	-355 / 0	-84.3	-84.3	0.08 (1)	6.25
C-J	-355 / 0	-84.3	-84.3	0.08 (1)	6.25
J-D	-412 / 0	-84.3	-84.3	0.05 (1)	6.25
D-E	0 / 17	-84.3	-84.3	0.11 (1)	10.00
B-G	0 / 305	-28.0	-28.0	0.08 (1)	10.00
G-F	0 / 305	-28.0	-28.0	0.10 (1)	10.00
F-I	0 / 305	-28.0	-28.0	0.10 (1)	10.00
I-D	0 / 305	-28.0	-28.0	0.08 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F-C	0 / 165	0.04 (2)
G-H	-34 / 113	0.00 (1)
I-J	-34 / 113	0.00 (1)

DWG NO. TAM 45042-17

STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.11 (A-B:1) , BC=0.10 (F-G:1) , WB=0.04 (C-F:2) , SSI=0.09 (C-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (D) (INPUT = 0.90)
JSI METAL= 0.10 (D) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285876	T93Z	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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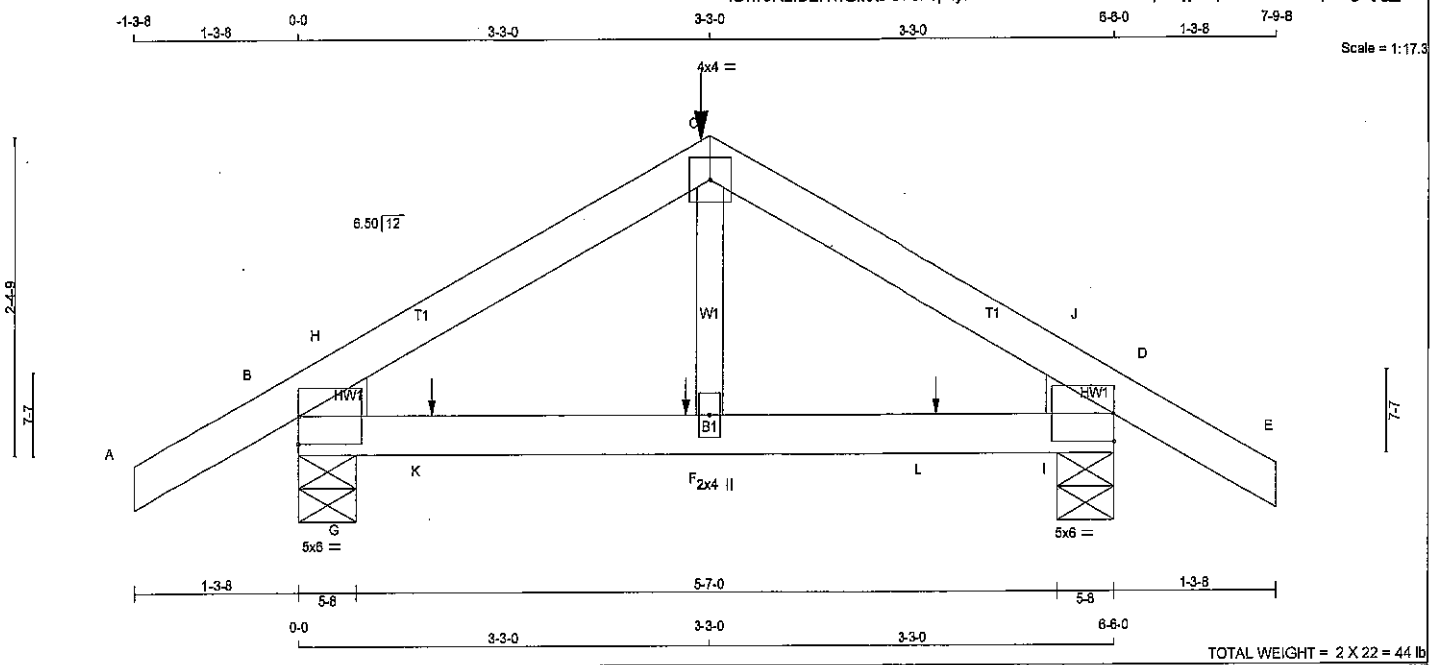

TOTAL WEIGHT = 2 X 22 = 44 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-I	MT20	5.0	6.0		
C	TTW-p	MT20	4.0	4.0		
D	TMBH1-I	MT20	5.0	6.0		
F	BMW+w	MT20	2.0	4.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 2.4 lbs FACTORED DOWN AT 3-3-0 ON TOP CHORD, AND AT 1-0-12, AND AT 3-0-12, AND AT 5-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
B	481	0	481	0	0	5-8	5-8	2x4 L
D	481	0	481	0	0	5-8	5-8	2x4 R

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	378	236 / 0	66 / 0	0 / 0	0 / 0	74 / 0	0 / 0
D	378	236 / 0	66 / 0	0 / 0	0 / 0	74 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 17	-84.3	-84.3	0.12 (1)	10.00	
B-H	-414 / 0	-84.3	-84.3	0.07 (1)	6.25	
H-C	-357 / 0	-84.3	-84.3	0.10 (1)	6.25	
C-J	-357 / 0	-84.3	-84.3	0.10 (1)	6.25	
J-D	-414 / 0	-84.3	-84.3	0.07 (1)	6.25	
D-E	0 / 17	-84.3	-84.3	0.12 (1)	10.00	
B-G	0 / 307	-28.0	-28.0	0.09 (1)	10.00	
G-K	0 / 307	-28.0	-28.0	0.11 (1)	10.00	
K-F	0 / 307	-28.0	-28.0	0.11 (1)	10.00	
F-L	0 / 307	-28.0	-28.0	0.11 (1)	10.00	
L-I	0 / 307	-28.0	-28.0	0.11 (1)	10.00	
I-D	0 / 307	-28.0	-28.0	0.09 (1)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F-C	0 / 165	0.04 (2)
G-H	-33 / 114	0.00 (1)
I-J	-33 / 114	0.00 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE
C	3-3-0	-2	-2	---	FRONT	VERT	TOTAL
F	3-0-12	---	---	---	FRONT	VERT	TOTAL
K	1-0-12	---	---	---	FRONT	VERT	TOTAL
L	5-0-12	---	---	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.12 (A-B:1) , BC=0.11 (F-G:1) , WB=0.04 (C-F:2) , SSI=0.10 (C-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (D) (INPUT = 0.90)
JSI METAL= 0.10 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 45043-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285876	T102	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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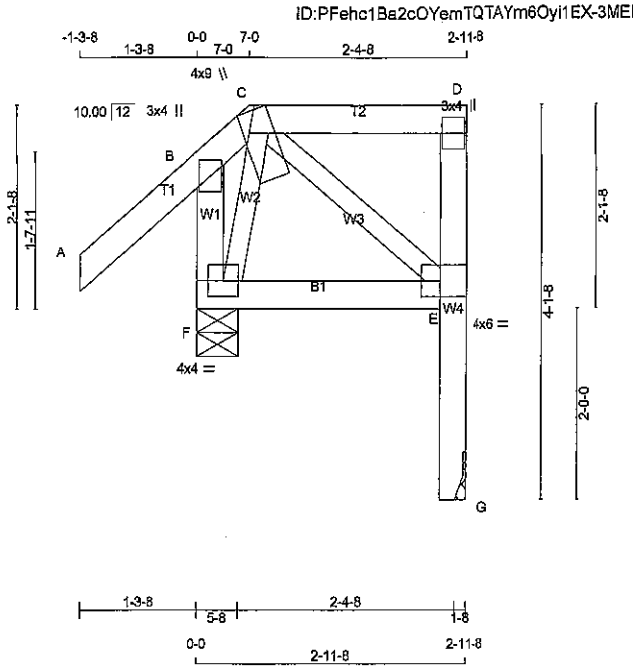

Scale: 1/2"=1"

TOTAL WEIGHT = 4 X 18 = 72 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
G - D	2x4	DRY	No.2	SPF
F - B	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TTWW+m	MT20	4.0	9.0	Edge	0.75
D	TMV+p	MT20	3.0	4.0		
E	BVMW-I	MT20	4.0	6.0	2.00	2.50
F	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT	REQRD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	BRG
G	139	0	139	0	0	IN-SX	IN-SX
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	
F	310	0	310	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	118	59 / -17	31 / 0	0 / 0	0 / 0	28 / 0	0 / 0
F	234	163 / 0	31 / 0	0 / 0	0 / 0	40 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS						WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)		MAX CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO		LENGTH	FR-TO		
A- B	0 / 37	-84.3	-84.3	0.12 (1)	10.00	F- C	0 / 105	0.02 (5)
B- C	-86 / 0	-84.3	-84.3	0.11 (1)	6.25	C- E	0 / 42	0.01 (5)
C- D	0 / 0	-84.3	-84.3	0.08 (1)	10.00			
G- E	-139 / 0	0.0	0.0	0.02 (1)	7.81			
E- D	-99 / 0	0.0	0.0	0.01 (1)	7.81			
F- B	-276 / 0	0.0	0.0	0.03 (1)	7.81			
F- E	-33 / 0	-28.0	-28.0	0.07 (3)	6.25			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.12 (A-B:1) , BC=0.07 (E-F:3) , WB=0.02 (C-F:5) , SSI=0.09 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.16 (F) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

DWG NO. TAM 45644-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285876	T103	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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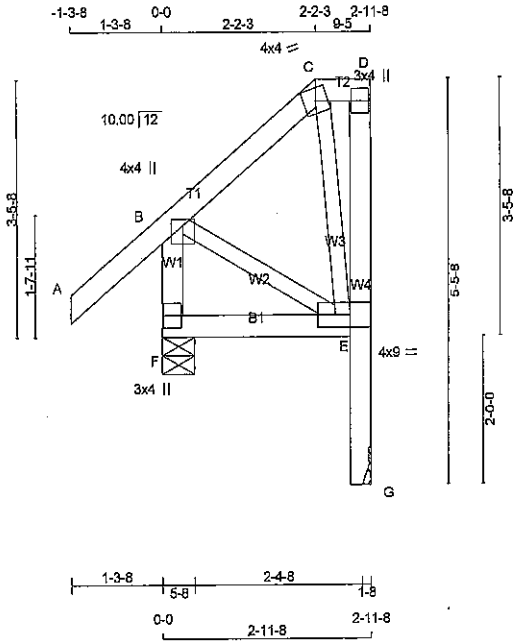

TOTAL WEIGHT = 4 X 21 = 84 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
G - D	2x4	DRY	No.2	SPF
F - B	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW-m	MT20	4.0	4.0		
D	TMV+p	MT20	3.0	4.0		
E	BVMWW-I	MT20	4.0	9.0	2.00	3.00
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	166	0	166	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
F	283	0	283	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	136	76 / 0	31 / 0	0 / 0	0 / 0	30 / 0	0 / 0
F	216	147 / 0	31 / 0	0 / 0	0 / 0	38 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF) CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 37	-84.3	-84.3	0.12 (1)	10.00	C- E	-106 / 0	0.02 (1)
B- C	-33 / 0	-84.3	-84.3	0.07 (1)	6.25	B- E	0 / 26	0.01 (1)
C- D	0 / 0	-84.3	-84.3	0.01 (1)	10.00			
G- E	-166 / 0	0.0	0.0	0.02 (1)	7.81			
E- D	-33 / 0	0.0	0.0	0.01 (1)	7.81			
F- B	-241 / 0	0.0	0.0	0.03 (1)	7.81			
F- E	0 / 0	-28.0	-28.0	0.07 (3)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.12 (A-B:1) , BC=0.07 (E-F:3) , WB=0.02 (C-E:1) , SSI=0.07 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1658

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)

DWG NO. TAM 45045-17

STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285876	T179	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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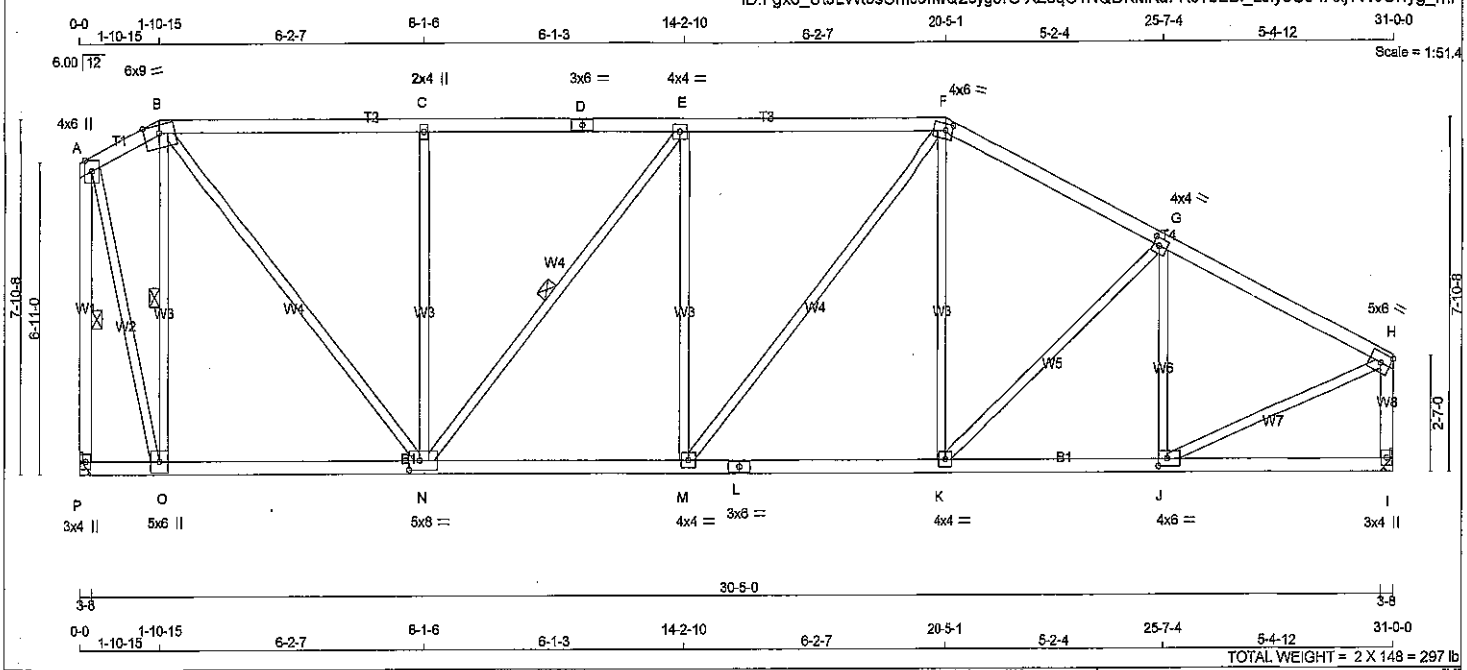


LUMBER					
N. L. G. A. RULES					
CHORDS	SIZE		LUMBER		DESCR.
A - B	2x4	DRY	No.2		SPF
B - D	2x4	DRY	No.2		SPF
D - F	2x4	DRY	No.2		SPF
F - H	2x4	DRY	No.2		SPF
P - A	2x4	DRY	No.2		SPF
I - H	2x4	DRY	No.2		SPF
P - L	2x4	DRY	No.2		SPF
L - I	2x4	DRY	No.2		SPF
ALL WEBS EXCEPT	2x3	DRY	No.2		SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
B	TTWW-m	MT20	6.0	9.0	Edge	
C	TMW+w	MT20	2.0	4.0		
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTWW-m	MT20	4.0	6.0	1.75	2.25
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
H	TMVW-t	MT20	5.0	6.0		Edge
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	4.0	6.0	2.00	2.50
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMWWW-t	MT20	5.0	8.0	2.50	3.00
O	BMWW+t	MT20	5.0	6.0		
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT	REQRD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	BRG
P	1740	0	1740	0	0	IN-SX	IN-SX
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-8	
I	1740	0	1740	0	0	HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1429	793 / 0	326 / 0	0 / 0	0 / 0	310 / 0	0 / 0
I	1429	793 / 0	326 / 0	0 / 0	0 / 0	310 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.34 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

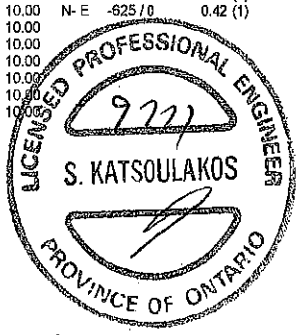
1 LATERAL BRACE(S) AT 1/2 LENGTH OF B-O, A-P, E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO						FR-TO			
A- B	-492 / 0	-84.3	-84.3	0.06 (1)	6.25	O- B	-1300 / 0	0.51 (1)	
B- C	-1427 / 0	-84.3	-84.3	0.51 (1)	4.83	K- F	0 / 276	0.06 (2)	
C- D	-1427 / 0	-84.3	-84.3	0.51 (1)	4.62	K- G	-28 / 7	0.03 (1)	
D- E	-1427 / 0	-84.3	-84.3	0.51 (1)	4.82	J- G	-526 / 0	0.22 (1)	
E- F	-1819 / 0	-84.3	-84.3	0.55 (1)	4.34	A- O	0 / 1504	0.34 (1)	
F- G	-1825 / 0	-84.3	-84.3	0.49 (1)	4.41	J- H	0 / 1768	0.40 (1)	
G- H	-1799 / 0	-84.3	-84.3	0.49 (1)	4.44	M- F	0 / 326	0.07 (1)	
P- A	-1740 / 0	0.0	0.0	0.39 (1)	5.05	B- N	0 / 1587	0.36 (1)	
I- H	-1677 / 0	0.0	0.0	0.23 (1)	6.41	M- E	-80 / 195	0.09 (1)	
						N- C	-565 / 0	0.66 (1)	
P- O	0 / 0	-28.0	-28.0	0.15 (3)	10.00	N- E	-625 / 0	0.42 (1)	
O- N	0 / 421	-28.0	-28.0	0.26 (2)	10.00				
N- M	0 / 1819	-28.0	-28.0	0.42 (2)	10.00				
M- L	0 / 1612	-28.0	-28.0	0.40 (2)	10.00				
L- K	0 / 1612	-28.0	-28.0	0.40 (2)	10.00				
K- J	0 / 1629	-28.0	-28.0	0.37 (2)	10.00				
J- I	0 / 0	-28.0	-28.0	0.18 (3)	10.00				



DWG NO. TAM 45046-17
STRUCTURAL COMPONENT ONLY

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.55 (E-F:1) , BC=0.42 (M-N:2) , WB=0.66 (C-N:1) , SSI=0.24 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)		SECTION (PLI)	
		MAX	MIN	MAX	MIN
MT20	MAX	618	354	1667	622
	MIN	2284	1656		

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (N) (INPUT = 0.90)
JSI METAL= 0.47 (H) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285876	T180	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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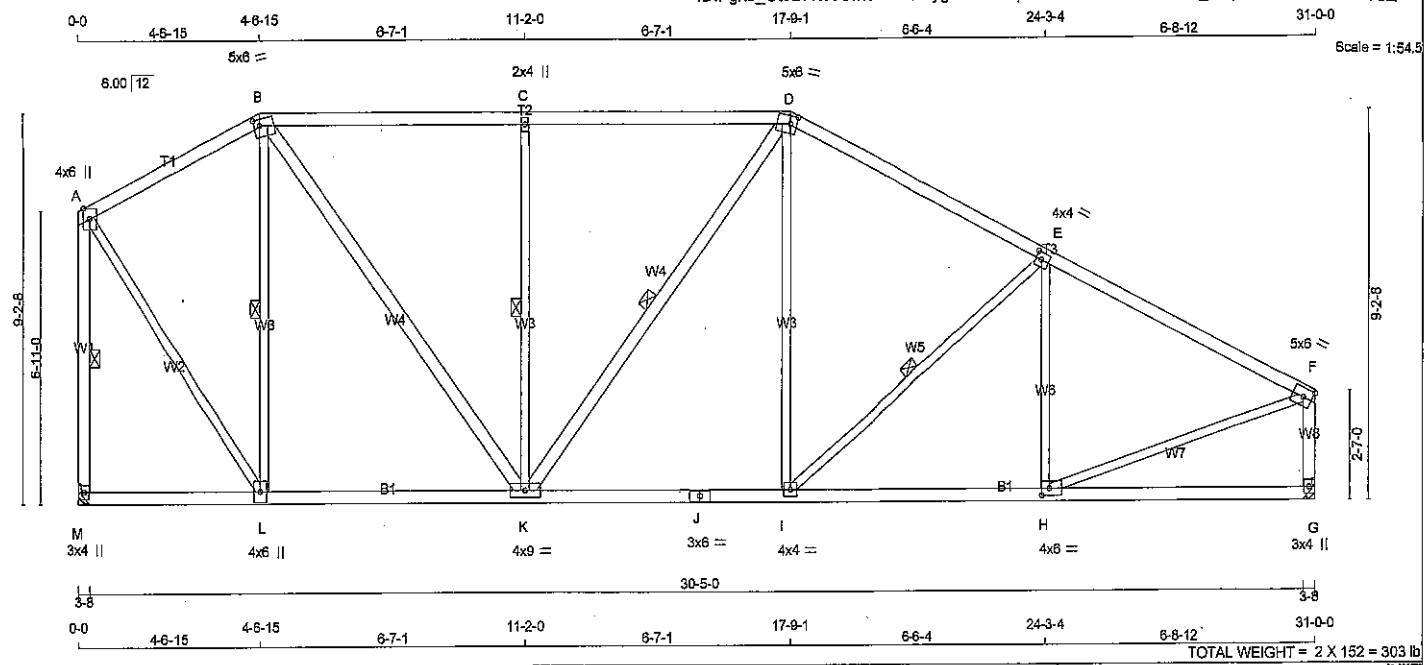


TOTAL WEIGHT = 2 X 152 = 303 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
M - A	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
B - K	2x4	DRY	No.2	SPF
K - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
B	TTWW-m	MT20	5.0	6.0	1.75	1.75
C	TMW+w	MT20	2.0	4.0		
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0	2.00	1.75
F	TMW-t	MT20	5.0	6.0		Edge
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	4.0	6.0	2.00	2.25
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	9.0		
L	BMWW+t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	
M	1740	0	1740	0	0			HANGER BY OTHERS MIN. SEAT SIZE: 3-8
G	1740	0	1740	0	0			HANGER BY OTHERS MIN. SEAT SIZE: 3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1429	793 / 0	326 / 0	0 / 0	0 / 0	310 / 0	0 / 0
G	1429	793 / 0	326 / 0	0 / 0	0 / 0	310 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.24 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF B-L, C-K, D-K, E-I, A-M.

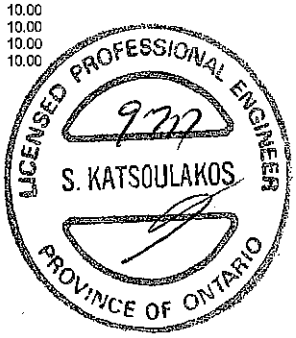
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-878 / 0	-84.3	-84.3	0.24 (1)		6.25
B- C	-1455 / 0	-84.3	-84.3	0.51 (1)		4.79
C- D	-1455 / 0	-84.3	-84.3	0.51 (1)		4.79
D- E	-1696 / 0	-84.3	-84.3	0.54 (1)		4.48
E- F	-1900 / 0	-84.3	-84.3	0.58 (1)		4.24
M- A	-1693 / 0	0.0	0.0	0.38 (1)		5.11
G- F	-1663 / 0	0.0	0.0	0.22 (1)		6.43
M- L	0 / 0	-28.0	-28.0	0.21 (2)		10.00
L- K	0 / 773	-28.0	-28.0	0.33 (2)		10.00
K- J	0 / 1493	-28.0	-28.0	0.41 (2)		10.00
J- I	0 / 1493	-28.0	-28.0	0.41 (2)		10.00
I- H	0 / 1726	-28.0	-28.0	0.51 (2)		10.00
H- G	0 / 0	-28.0	-28.0	0.31 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- B	-932 / 0	0.52 (1)
B- K	0 / 1146	0.18 (1)
K- C	-886 / 0	0.38 (1)
K- D	-75 / 0	0.05 (2)
I- D	0 / 446	0.10 (2)
I- E	-312 / 0	0.18 (1)
H- E	-375 / 79	0.21 (1)
A- L	0 / 1356	0.31 (1)
H- F	0 / 1822	0.41 (1)



DWG NO. TAM 45047-17
STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.58 (E-F:1) , BC=0.51 (H-I:2) , WB=0.52 (B-L:1) , SSI=0.27 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (F) (INPUT = 0.90)
JSI METAL= 0.50 (F) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285876	T181	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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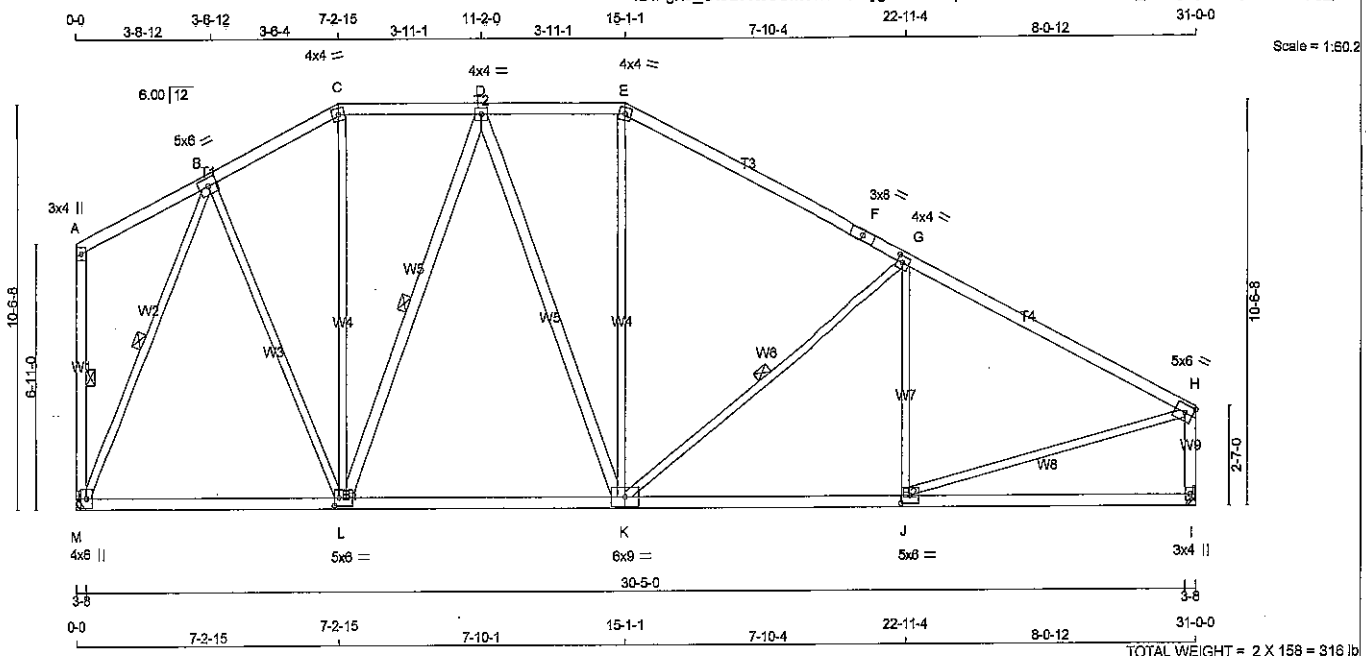


TOTAL WEIGHT = 2 X 158 = 316 lb

[M](F)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
M - A	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
L - D	2x4	DRY	No.2	SPF
D - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	5.0	6.0		
C	TTW-m	MT20	4.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TTW-m	MT20	4.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
H	TMVW-t	MT20	5.0	6.0		Edge
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	5.0	6.0	2.50	2.75
K	BSWWW-t	MT20	6.0	9.0		
L	BMWWW-t	MT20	5.0	6.0	2.50	1.50
M	BMVW1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
M	1740	0	1740	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	
I	1740	0	1740	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1429	793 / 0	326 / 0	0 / 0	0 / 0	310 / 0	0 / 0
I	1429	793 / 0	326 / 0	0 / 0	0 / 0	310 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.61 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

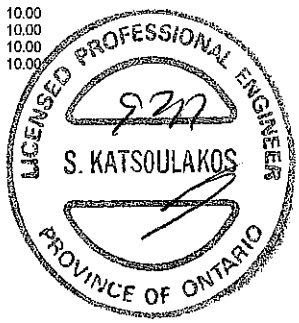
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-L, G-K, A-M, B-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 15	-84.3	-84.3	0.17 (1)	10.00	B-L	0 / 741	0.17 (1)
B-C	-1095 / 0	-84.3	-84.3	0.14 (1)	5.89	L-C	0 / 283	0.06 (2)
C-D	-987 / 0	-84.3	-84.3	0.17 (1)	6.13	L-D	-727 / 0	0.48 (1)
D-E	-1328 / 0	-84.3	-84.3	0.18 (1)	5.44	D-K	0 / 299	0.05 (1)
E-F	-1522 / 0	-84.3	-84.3	0.80 (1)	4.13	K-E	0 / 342	0.08 (2)
F-G	-1522 / 0	-84.3	-84.3	0.80 (1)	4.13	K-G	-577 / 0	0.44 (1)
G-H	-1952 / 0	-84.3	-84.3	0.89 (1)	3.61	J-G	-250 / 194	0.18 (1)
M-A	-121 / 0	0.0	0.0	0.03 (1)	6.25	M-B	-1677 / 0	0.97 (1)
I-H	-1649 / 0	0.0	0.0	0.22 (1)	6.45	J-H	0 / 1847	0.42 (1)
M-L	0 / 669	-28.0	-28.0	0.47 (2)	10.00			
L-K	0 / 1221	-28.0	-28.0	0.54 (2)	10.00			
K-J	0 / 1778	-28.0	-28.0	0.65 (2)	10.00			
J-I	0 / 0	-28.0	-28.0	0.45 (3)	10.00			



DWG NO. TAM 45048-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.28")

CSI: TC=0.89 (G-H:1) , BC=0.65 (J-K:2) , WB=0.97 (B-M:1) , SSI=0.29 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (J) (INPUT = 0.90)
JSI METAL= 0.52 (H) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285876	T182	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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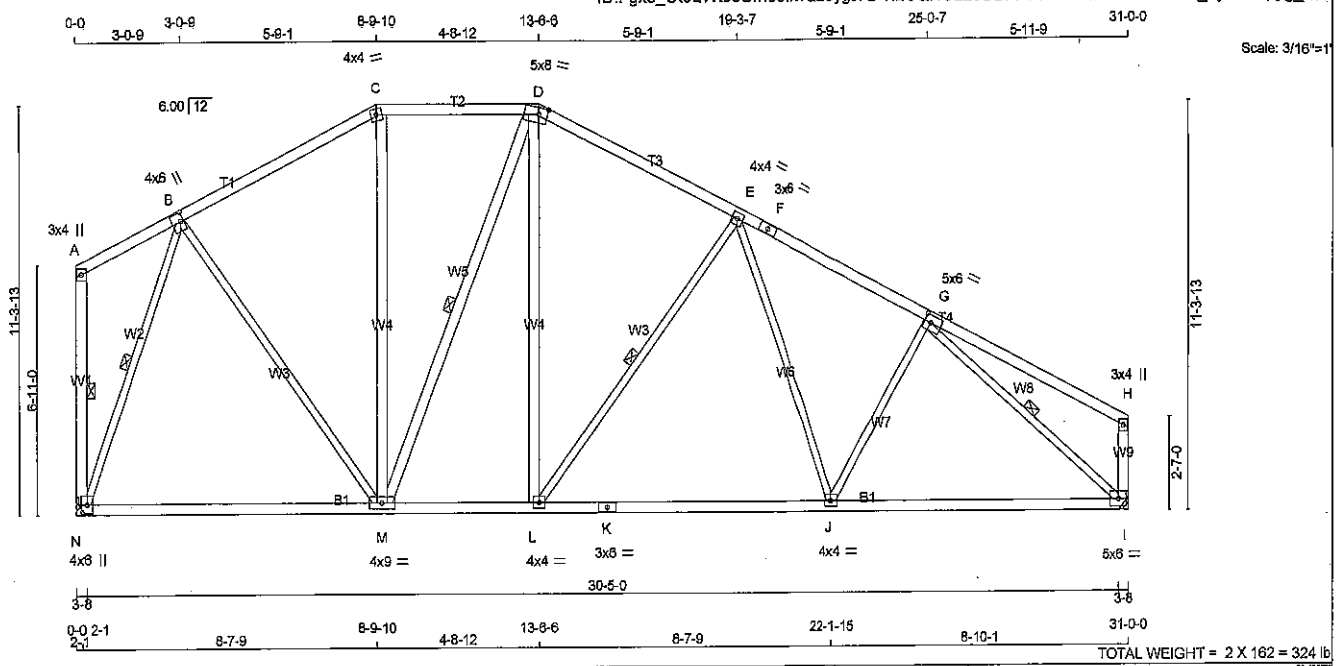

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
N - A	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
M - C	2x4	DRY	No.2	SPF
M - D	2x4	DRY	No.2	SPF
L - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW+t	MT20	4.0	6.0	3.00	1.00
C	TTW-m	MT20	4.0	4.0		
D	TTWW-m	MT20	5.0	8.0	2.25	3.00
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	5.0	6.0	2.50	2.25
H	TMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	5.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMWWW-t	MT20	4.0	9.0		
N	BMVW1+p	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1740	0	1740	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	
I	1740	0	1740	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1429	793 / 0	326 / 0	0 / 0	0 / 0	310 / 0	0 / 0
I	1429	793 / 0	326 / 0	0 / 0	0 / 0	310 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF B-N, D-M, E-L, A-N, G-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 36	-84.3	-84.3	0.34 (1)	10.00	N- B	-1697 / 0	0.86 (1)
B- C	-1159 / 0	-84.3	-84.3	0.34 (1)	5.51	B- M	0 / 720	0.16 (1)
C- D	-1020 / 0	-84.3	-84.3	0.25 (1)	5.90	M- C	0 / 231	0.04 (3)
D- E	-1388 / 0	-84.3	-84.3	0.36 (1)	5.16	M- D	-500 / 0	0.40 (1)
E- F	-1861 / 0	-84.3	-84.3	0.38 (1)	4.58	L- D	0 / 672	0.11 (1)
F- G	-1861 / 0	-84.3	-84.3	0.36 (1)	4.58	L- E	-653 / 0	0.48 (1)
G- H	0 / 21	-84.3	-84.3	0.38 (1)	10.00	E- J	0 / 347	0.08 (2)
N- A	-47 / 0	0.0	0.0	0.01 (1)	6.25	J- G	0 / 229	0.05 (3)
I- H	-200 / 0	0.0	0.0	0.03 (1)	7.81	G- I	-2173 / 0	0.88 (1)
N- M	0 / 596	-28.0	-28.0	0.53 (2)	10.00			
M- L	0 / 1212	-28.0	-28.0	0.61 (2)	10.00			
L- K	0 / 1587	-28.0	-28.0	0.77 (2)	10.00			
K- J	0 / 1587	-28.0	-28.0	0.77 (2)	10.00			
J- I	0 / 1625	-28.0	-28.0	0.77 (2)	10.00			

DWG NO. T&M 45049-17
STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.26")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 858 (0.43")

CSI: TC=0.38 (G-H:1) , BC=0.77 (I-J:2) , WB=0.86 (G-I:1) , SSI=0.21 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.56 (K) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285864	T183S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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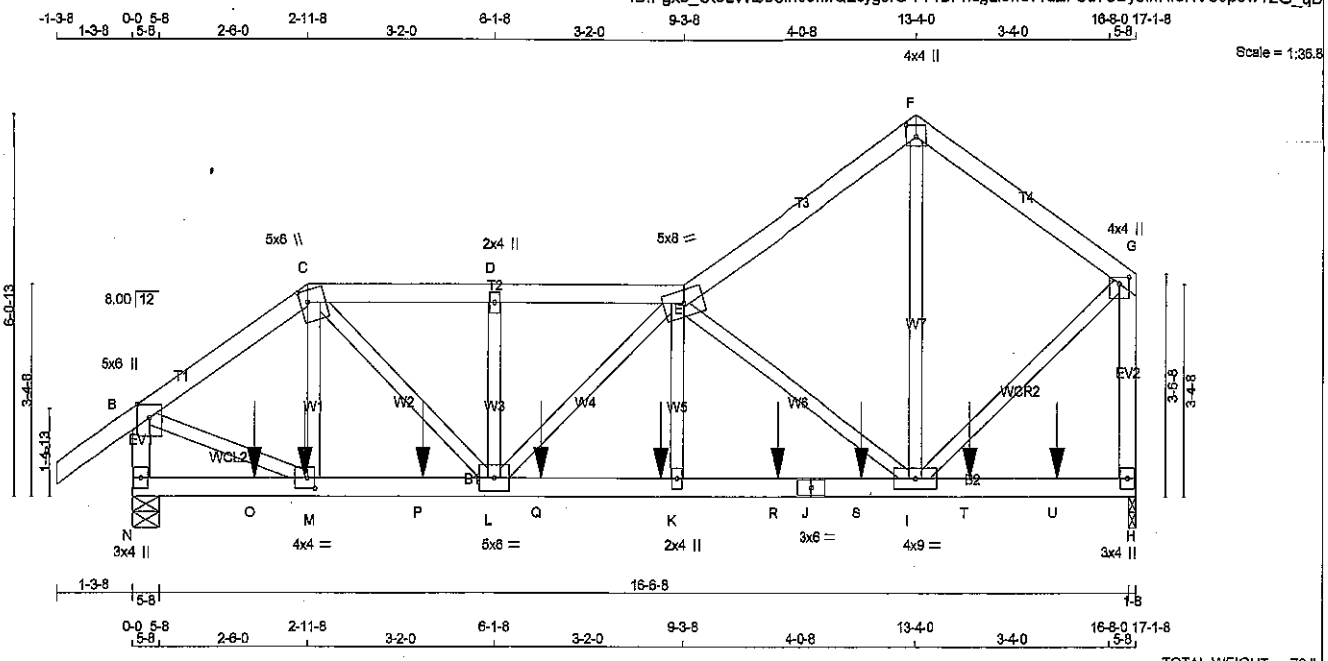


TOTAL WEIGHT = 76 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWWW-m	MT20	5.0	8.0		
F	TTW+p	MT20	4.0	4.0	2.25	2.00
G	TMVW+p	MT20	4.0	4.0	1.25	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BS-t	MT20	3.0	6.0		
K	BMW+w	MT20	2.0	4.0		
L	BMWWW-t	MT20	5.0	6.0		
M	BMWW-t	MT20	4.0	4.0	2.00	1.50
N	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 14.8 lbs FACTORED DOWN AT 2-0-12, 14.8 lbs FACTORED DOWN AT 2-11-0, 14.8 lbs FACTORED DOWN AT 4-11-0, 14.8 lbs FACTORED DOWN AT 6-11-0, 14.8 lbs FACTORED DOWN AT 8-11-0, 110.7 lbs FACTORED DOWN AT 10-10-12, 138.0 lbs FACTORED DOWN AT 12-4-12, AND 138.0 lbs FACTORED DOWN AT 14-3-4, AND 110.7 lbs FACTORED DOWN AT 15-9-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1216	0	1216	0	0	5-8	5-8
H	1361	0	1361	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	986	568 / 0	211 / 0	0 / 0	0 / 0	207 / 0	0 / 0
H	1092	648 / 0	221 / 0	0 / 0	0 / 0	223 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.15 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI	MAX (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 32	-84.3	-84.3	0.12 (1)	10.00		M- C	-200 / 36	0.04 (1)
B- C	-1107 / 0	-84.3	-84.3	0.16 (1)	5.81		C- L	0 / 833	0.21 (1)
C- D	-1510 / 0	-84.3	-84.3	0.16 (1)	5.15		L- D	-308 / 0	0.07 (1)
D- E	-1510 / 0	-84.3	-84.3	0.16 (1)	5.15		L- E	-291 / 0	0.09 (1)
E- F	-866 / 0	-84.3	-84.3	0.28 (1)	6.18		K- E	0 / 237	0.06 (2)
F- G	-867 / 0	-84.3	-84.3	0.25 (1)	6.23		E- I	-1244 / 0	0.55 (1)
N- B	-1181 / 0	0.0	0.0	0.13 (1)	7.31		I- F	0 / 630	0.16 (1)
H- G	-1254 / 0	0.0	0.0	0.27 (1)	7.14		B- M	0 / 974	0.24 (1)
							I- G	0 / 947	0.23 (1)
N- O	0 / 0	-28.0	-28.0	0.08 (2)	10.00				
O- M	0 / 0	-28.0	-28.0	0.08 (2)	10.00				
M- P	0 / 912	-28.0	-28.0	0.21 (1)	10.00				
P- L	0 / 912	-28.0	-28.0	0.21 (1)	10.00				
L- Q	0 / 1719	-28.0	-28.0	0.39 (1)	10.00				
Q- K	0 / 1719	-28.0	-28.0	0.39 (1)	10.00				
K- R	0 / 1713	-28.0	-28.0	0.59 (1)	10.00				
R- J	0 / 1713	-28.0	-28.0	0.59 (1)	10.00				
J- S	0 / 1713	-28.0	-28.0	0.59 (1)	10.00				
S- I	0 / 1713	-28.0	-28.0	0.59 (1)	10.00				
I- T	0 / 0	-28.0	-28.0	0.29 (1)	10.00				
T- U	0 / 0	-28.0	-28.0	0.29 (1)	10.00				
U- H	0 / 0	-28.0	-28.0	0.29 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
K	8-11-0	-8	-15	—	FRONT	VERT	TOTAL
M	2-11-0	-8	-15	—	FRONT	VERT	TOTAL
O	2-0-12	-8	-15	—	FRONT	VERT	TOTAL
P	4-11-0	-8	-15	—	FRONT	VERT	TOTAL
Q	6-11-0	-8	-15	—	FRONT	VERT	TOTAL
R	10-10-12	-111	-111	—	FRONT	VERT	TOTAL
S	12-4-12	-138	-138	—	FRONT	VERT	TOTAL
T	14-3-4	-138	-138	—	FRONT	VERT	TOTAL
U	15-9-4	-111	-111	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.57")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.57")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.28/1.00 (E-F:1) , BC=0.59/1.00 (I-K:1) , WB=0.55/1.00 (E-I:1) , SSI=0.21/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

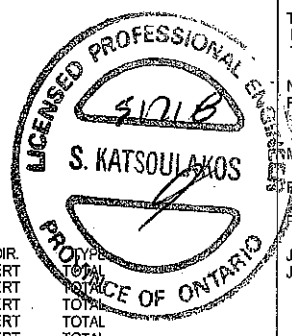
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.62 (J) (INPUT = 1.00)



DWG NO. TAM26665-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285864	T184	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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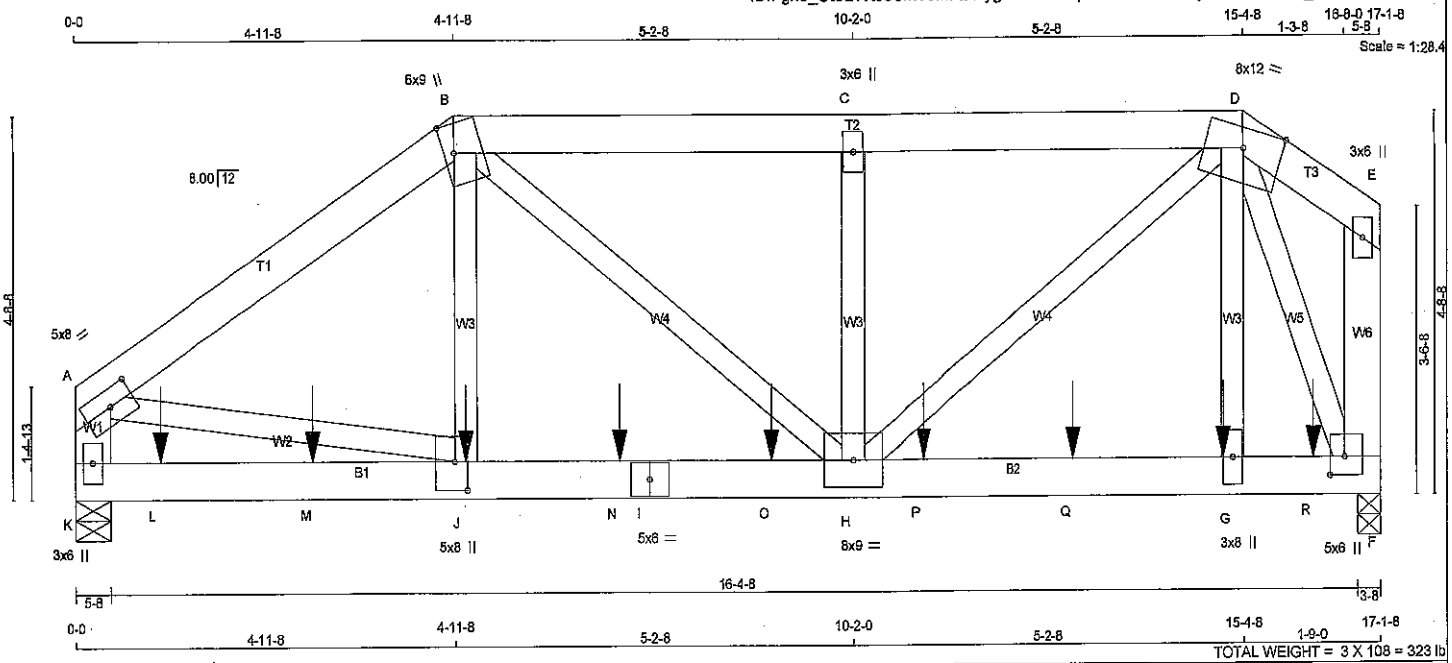

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x6	DRY	No.2	SPF
B - D	2x6	DRY	No.2	SPF
D - E	2x6	DRY	No.2	SPF
K - A	2x6	DRY	No.2	SPF
F - E	2x6	DRY	No.2	SPF
K - I	2x6	DRY	2100F 1.8E	SPF
I - F	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- B	2	12	TOP
B- D	2	12	TOP
D- E	2	12	TOP
K- A	2	12	TOP
F- E	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
K- I	2	8	SIDE(494.5)
I- F	2	8	SIDE(582.9)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	8.0	2.50	3.75
B	TTWW+m	MT20	6.0	9.0	4.25	1.50
C	TMW+w	MT20	3.0	6.0		
D	TTWWW-m	MT20	8.0	12.0	Edge	
E	TMV+p	MT20	3.0	6.0		
F	BMVW1+p	MT20	5.0	6.0	2.75	2.25
G	BMW+w	MT20	3.0	8.0		
H	BMWWW-t	MT20	8.0	9.0		
I	BS-t	MT20	5.0	8.0		
J	BMWW+t	MT20	5.0	8.0	4.25	2.00
K	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
K	6529	0	6529	0	0	5-8	5-8
F	9145	0	9145	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEA0	SOIL
K	6912	3985 / 0	1476 / 0	0 / 0	0 / 0	1451 / 0	0 / 0
F	7444	4238 / 0	1626 / 0	0 / 0	0 / 0	1581 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-9228 / 0	-84.3	-84.3	0.11 (1)	4.75	
B- C	-8719 / 0	-84.3	-84.3	0.10 (1)	4.86	
C- D	-8719 / 0	-84.3	-84.3	0.10 (1)	4.86	
D- E	0 / 0	-84.3	-84.3	0.01 (1)	10.00	
K- A	-8698 / 0	0.0	0.0	0.14 (1)	6.84	
F- E	-74 / 0	0.0	0.0	0.00 (1)	7.81	
K- L	0 / 0	-28.0	-28.0	0.24 (1)	10.00	
L- M	0 / 0	-28.0	-28.0	0.24 (1)	10.00	
M- J	0 / 0	-28.0	-28.0	0.24 (1)	10.00	
J- N	0 / 7798	-28.0	-28.0	0.36 (1)	10.00	
N- I	0 / 7798	-28.0	-28.0	0.36 (1)	10.00	
I- O	0 / 7798	-28.0	-28.0	0.36 (1)	10.00	
O- H	0 / 7798	-28.0	-28.0	0.36 (1)	10.00	
H- P	0 / 3704	-28.0	-28.0	0.28 (1)	10.00	
P- Q	0 / 3704	-28.0	-28.0	0.28 (1)	10.00	
Q- G	0 / 3704	-28.0	-28.0	0.28 (1)	10.00	
G- R	0 / 3559	-28.0	-28.0	0.17 (1)	10.00	
R- F	0 / 3559	-28.0	-28.0	0.17 (1)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- B	0 / 3942	0.21 (1)
B- H	0 / 1185	0.06 (1)
H- C	-381 / 31	0.03 (1)
H- D	0 / 6457	0.35 (1)
G- D	0 / 4205	0.23 (1)
A- J	0 / 7779	0.42 (1)
D- F	-9300 / 0	0.66 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
G	15-1-4	-1712	-1712	—	BACK	VERT	TOTAL
J	5-1-4	-1826	-1826	—	BACK	VERT	TOTAL
L	1-1-4	-1826	-1826	—	BACK	VERT	TOTAL
M	3-1-4	-1826	-1826	—	BACK	VERT	TOTAL
N	7-1-4	-1712	-1712	—	BACK	VERT	TOTAL
O	9-1-4	-1712	-1712	—	BACK	VERT	TOTAL
P	11-1-4	-1712	-1712	—	BACK	VERT	TOTAL
Q	13-1-4	-1712	-1712	—	BACK	VERT	TOTAL
R	16-3-4	-1714	-1714	—	BACK	VERT	TOTAL

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.57")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.57")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.14/1.00 (A-K:1) , BC=0.36/1.00 (H-J:1) , WB=0.66/1.00 (D-F:1) , SSI=0.47/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (J) (INPUT = 0.80)
JSI METAL= 0.88 (J) (INPUT = 1.00)

PROFESSIONAL ENGINEER
S. KOTSOULAKOS
PROVINCE OF ONTARIO
1/24/18

DWG NO. TAM 5092-18
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285864	T184	1	3	TRUSS DESC.	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1826.2 lbs FACTORED DOWN AT 1-1-4, 1826.2 lbs FACTORED DOWN AT 3-1-4, 1826.2 lbs FACTORED DOWN AT 5-1-4, 1711.9 lbs FACTORED DOWN AT 7-1-4, 1711.9 lbs FACTORED DOWN AT 9-1-4, 1711.9 lbs FACTORED DOWN AT 11-1-4, 1711.9 lbs FACTORED DOWN AT 13-1-4, AND 1711.9 lbs FACTORED DOWN AT 15-1-4, AND 1714.0 lbs FACTORED DOWN AT 16-3-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
12/18
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM5092-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285862	T570	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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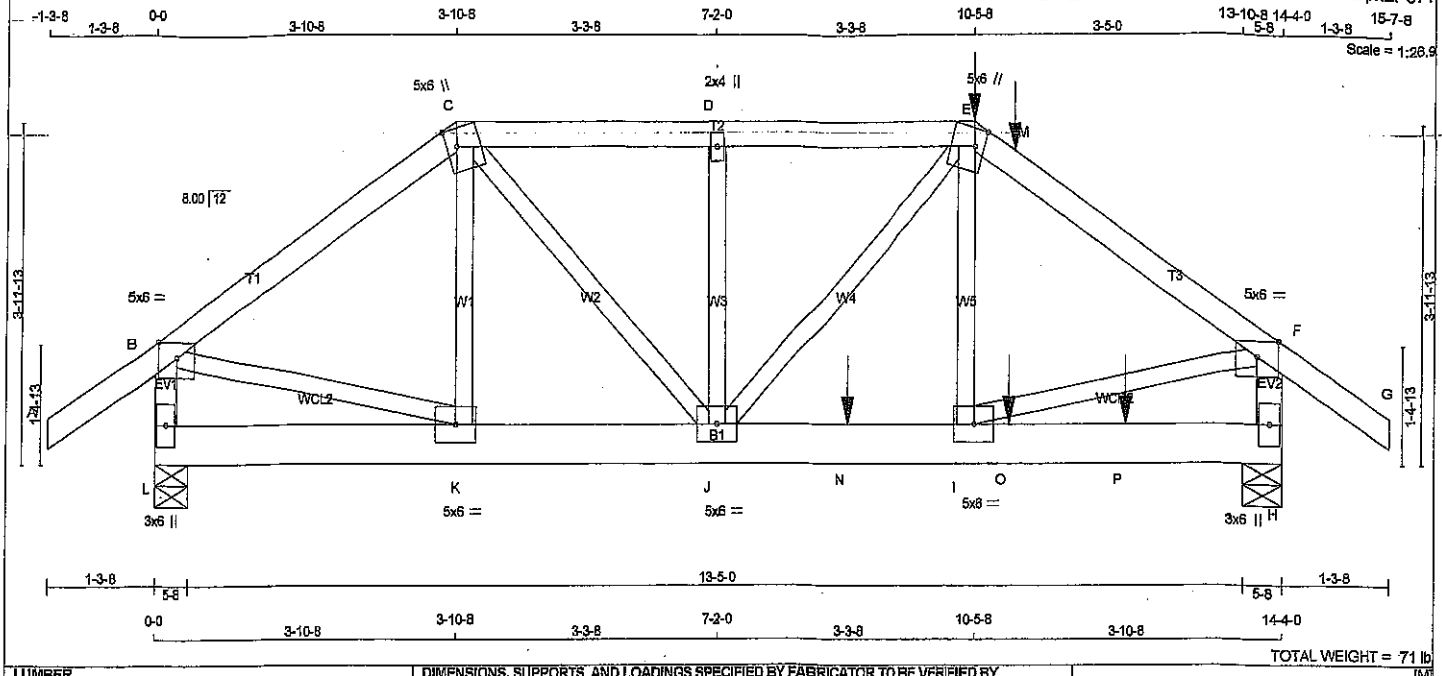

TOTAL WEIGHT = 71 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TTVW+m	MT20	5.0	6.0	2.50	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTVW+m	MT20	5.0	6.0	2.50	1.50
F	TMVW-p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	6.0		
I	BMWW-t	MT20	5.0	6.0		
J	BMWWW-t	MT20	5.0	6.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 183.4 lbs FACTORED DOWN AT 10-5-8, AND 62.0 lbs FACTORED DOWN AT 10-10-12 ON TOP CHORD, AND 587.8 lbs FACTORED DOWN AT 8-9-8, AND 33.0 lbs FACTORED DOWN AT 10-10-12, AND 41.0 lbs FACTORED DOWN AT 12-4-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1220	0	1220	0	0	5-8	5-8
H	1496	0	1496	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	972	587 / 0	190 / 0	0 / 0	0 / 0	195 / 0	0 / 0
H	1190	723 / 0	230 / 0	0 / 0	0 / 0	238 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.95 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 32	-84.3	-84.3	0.12 (1)	10.00	K- C	-171 / 89	0.04 (1)
B- C	-1158 / 0	-84.3	-84.3	0.27 (1)	5.56	C- J	0 / 727	0.18 (1)
C- D	-1446 / 0	-84.3	-84.3	0.17 (1)	5.22	J- D	-321 / 0	0.08 (1)
D- E	-1448 / 0	-84.3	-84.3	0.17 (1)	5.22	J- E	0 / 250	0.06 (1)
E- M	-1521 / 0	-84.3	-84.3	0.31 (1)	4.95	I- E	0 / 257	0.06 (2)
M- F	-1521 / 0	-84.3	-84.3	0.31 (1)	4.95	B- K	0 / 992	0.25 (1)
F- G	0 / 32	-84.3	-84.3	0.12 (1)	10.00	I- F	0 / 1314	0.33 (1)
L- B	-1165 / 0	0.0	0.0	0.13 (1)	7.35			
H- F	-1480 / 0	0.0	0.0	0.16 (1)	6.73			
L- K	0 / 0	-28.0	-28.0	0.06 (2)	10.00			
K- J	0 / 957	-28.0	-28.0	0.20 (1)	10.00			
J- N	0 / 1279	-28.0	-28.0	0.45 (1)	10.00			
N- I	0 / 1279	-28.0	-28.0	0.45 (1)	10.00			
I- O	0 / 0	-28.0	-28.0	0.11 (2)	10.00			
O- P	0 / 0	-28.0	-28.0	0.11 (2)	10.00			
P- H	0 / 0	-28.0	-28.0	0.11 (2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
E	10-5-8	-183	-183	---	FRONT	VERT	TOTAL
M	10-10-12	-62	-62	---	FRONT	VERT	TOTAL
N	8-9-8	-588	-588	---	FRONT	VERT	TOTAL
O	10-10-12	-19	-33	---	FRONT	VERT	TOTAL
P	12-4-12	-23	-41	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.48")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.48")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.31/1.00 (E-F:1) , BC=0.45/1.00 (I-J:1) , WB=0.33/1.00 (F-I:1) , SSI=0.23/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (B) (INPUT = 0.90)
JSI METAL= 0.32 (I) (INPUT = 1.00)

DWG NO. TAM 2121-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285862	T580	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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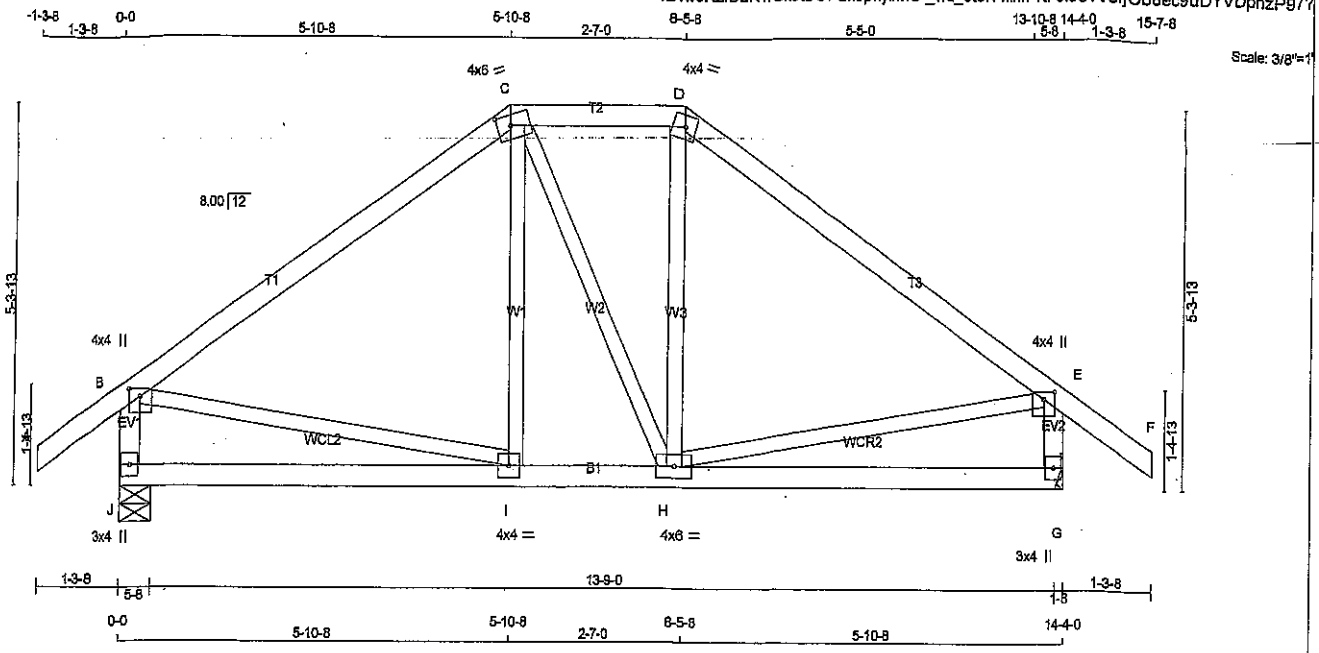

TOTAL WEIGHT = 63 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTWW-m	MT20	4.0	6.0	1.75	2.50
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	4.0	1.25	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	920	0	920	0	0	5-8	5-8
G	920	0	920	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	739	437 / 0	151 / 0	0 / 0	0 / 0	152 / 0	0 / 0
G	739	437 / 0	151 / 0	0 / 0	0 / 0	152 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00
B- C	-888 / 0	-84.3	-84.3	0.38 (1)	6.25
C- D	-574 / 0	-84.3	-84.3	0.08 (1)	6.25
D- E	-689 / 0	-84.3	-84.3	0.38 (1)	6.25
E- F	0 / 32	-84.3	-84.3	0.11 (1)	10.00
J- B	-849 / 0	0.0	0.0	0.09 (1)	7.81
G- E	-849 / 0	0.0	0.0	0.09 (1)	7.81
J- I	0 / 0	-28.0	-28.0	0.21 (2)	10.00
I- H	0 / 573	-28.0	-28.0	0.27 (2)	10.00
H- G	0 / 0	-28.0	-28.0	0.21 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- C	0 / 169	0.04 (3)
C- H	0 / 5	0.00 (3)
H- D	0 / 173	0.04 (3)
B- I	0 / 581	0.13 (1)
H- E	0 / 582	0.13 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.46")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.46")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.38/1.00 (D-E:1) , BC=0.27/1.00 (H-I:2) , WB=0.13/1.00 (E-H:1) , SSI=0.16/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1655

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (H) (INPUT = 0.90)
JSI METAL= 0.21 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4196
S. KATSOULAROS
PROVINCE OF ONTARIO

DWG NO. TAM2/2/12-T8
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285862	T590	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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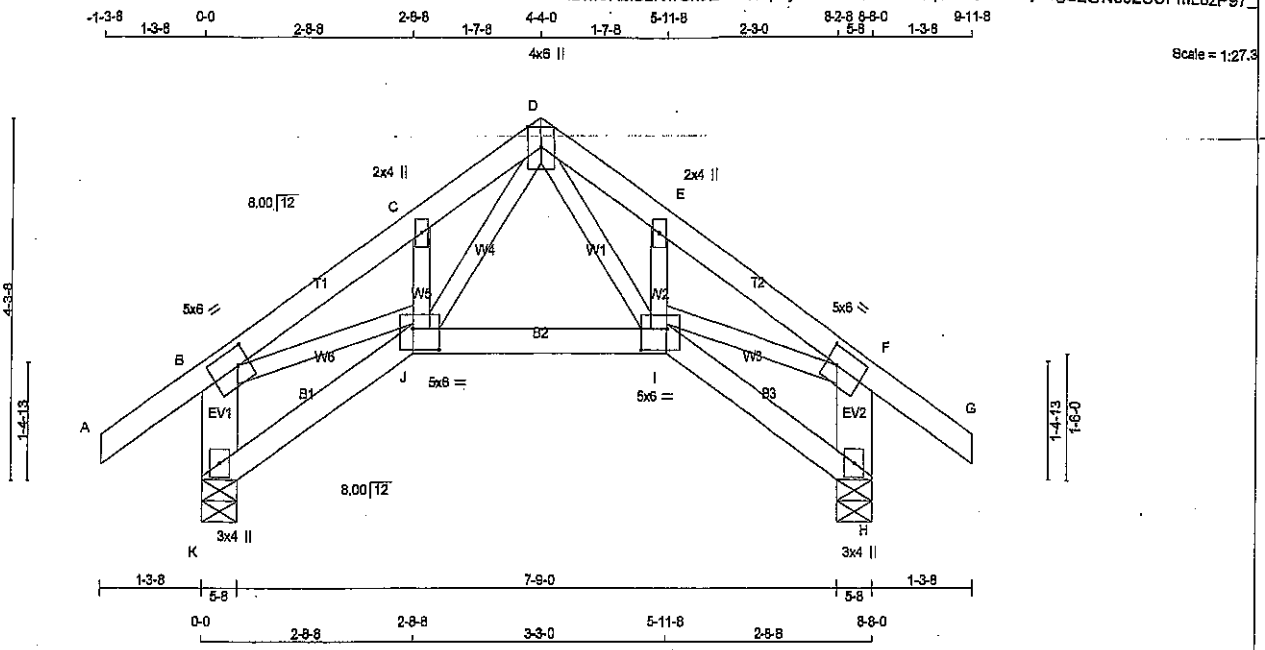

TOTAL WEIGHT = 2 X 42 = 84 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
K - B	2x6	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	1.75
C	TMW+w	MT20	2.0	4.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	5.0	6.0	2.50	1.75
H	BMV1+p	MT20	3.0	4.0		
I	BBWWW-p	MT20	5.0	6.0	3.00	4.00
J	BBWWW-p	MT20	5.0	6.0	3.00	4.00
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	602	0	602	0	0	5-8	5-8
H	602	0	602	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	478	292 / 0	91 / 0	0 / 0	0 / 0	95 / 0	0 / 0
H	478	292 / 0	91 / 0	0 / 0	0 / 0	95 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- B	-564 / 0	0.0	0.0	0.04 (1)	7.81	D- I	0 / 480	0.11 (1)
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	I- E	-209 / 0	0.03 (1)
B- C	-808 / 0	-84.3	-84.3	0.09 (1)	6.25	I- F	0 / 685	0.15 (1)
C- D	-825 / 0	-84.3	-84.3	0.03 (1)	6.25	J- D	0 / 480	0.11 (1)
D- E	-825 / 0	-84.3	-84.3	0.03 (1)	6.25	J- C	-209 / 0	0.03 (1)
E- F	-808 / 0	-84.3	-84.3	0.09 (1)	6.25	B- J	0 / 685	0.15 (1)
F- G	0 / 32	-84.3	-84.3	0.11 (1)	10.00			
H- F	-564 / 0	0.0	0.0	0.04 (1)	7.81			
K- J	0 / 0	-28.0	-28.0	0.06 (3)	10.00			
J- I	0 / 414	-28.0	-28.0	0.13 (2)	10.00			
I- H	0 / 0	-28.0	-28.0	0.06 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.29")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.29")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.11/1.00 (F-G:1) , BC=0.13/1.00 (I-J:2) , WB=0.15/1.00 (B-J:1) , SSI=0.08/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1667	822	2284	1856

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.23 (B) (INPUT = 1.00)

DWG NO. TAM21213-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285862	T600	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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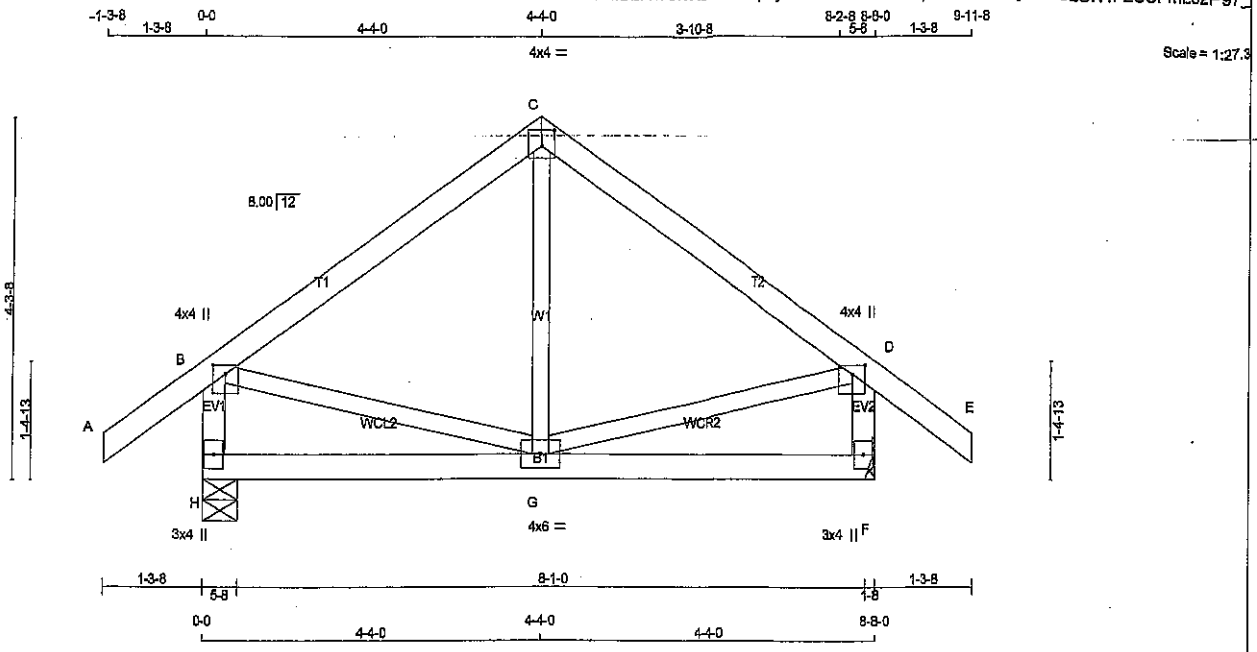

TOTAL WEIGHT = 37 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMVW+p	MT20	4.0	4.0	1.25	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	8.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	602	0	602	0	0	5-8	5-8
F	602	0	602	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	478	292 / 0	91 / 0	0 / 0	0 / 0	95 / 0	0 / 0
F	478	292 / 0	91 / 0	0 / 0	0 / 0	95 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	G- C	0 / 183	0.04 (3)
B- C	-337 / 0	-84.3	-84.3	0.20 (1)	6.25	B- G	0 / 289	0.07 (1)
C- D	-337 / 0	-84.3	-84.3	0.20 (1)	6.25	G- D	0 / 289	0.07 (1)
D- E	0 / 32	-84.3	-84.3	0.11 (1)	10.00			
H- B	-555 / 0	0.0	0.0	0.06 (1)	7.81			
F- D	-555 / 0	0.0	0.0	0.06 (1)	7.81			
H- G	0 / 0	-28.0	-28.0	0.15 (3)	10.00			
G- F	0 / 0	-28.0	-28.0	0.15 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.29")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.29")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.20/1.00 (C-D:1) , BC=0.15/1.00 (F-G:3) , WB=0.07/1.00 (B-G:1) , SSI=0.12/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.41 (G) (INPUT = 0.90)
JSI METAL= 0.12 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4918
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 2124-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285876	T620Z	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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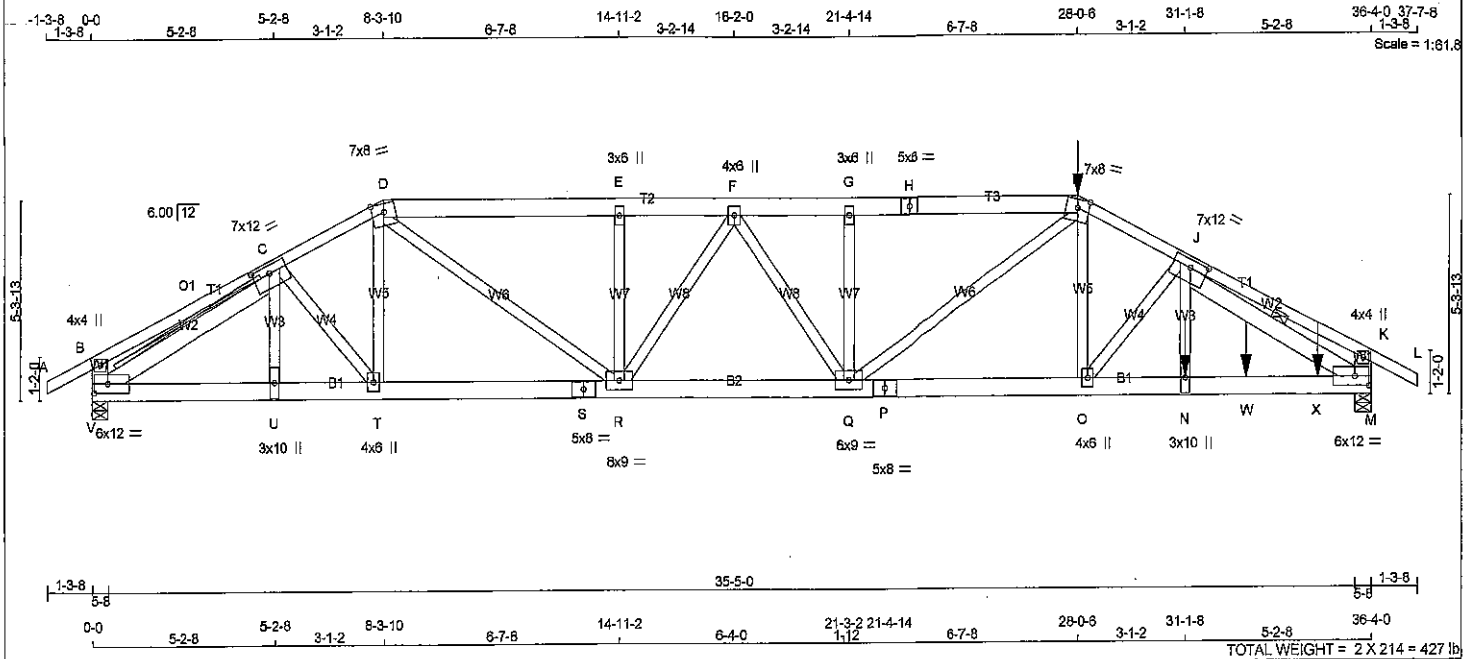

TOTAL WEIGHT = 2 X 214 = 427 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - H	2x6	DRY	No.2	SPF
H - I	2x6	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
V - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
V - S	2x6	DRY	2100F 1.8E	SPF
S - P	2x6	DRY	2100F 1.8E	SPF
P - M	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
V - C	2x6	DRY	No.2	SPF
J - M	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWWW-t	MT20	7.0	12.0	2.50	5.75
D	TTWW-m	MT20	7.0	8.0	Edge	
E	TMW+w	MT20	3.0	6.0		
F	TMWW+t	MT20	4.0	6.0		
G	TMW+w	MT20	3.0	6.0		
H	TS-t	MT20	5.0	8.0		
I	TTWW-m	MT20	7.0	8.0	Edge	
J	TMWWW-t	MT20	7.0	12.0	2.50	5.75
K	TMV+p	MT20	4.0	4.0		
M	BMVW1-t	MT20	6.0	12.0	3.00	4.75
N	BMW+w	MT20	3.0	10.0		
O	BMWW+t	MT20	4.0	6.0		
P	BS-t	MT20	5.0	8.0		
Q	BMWWW-t	MT20	6.0	9.0		
R	BMWWW-t	MT20	6.0	9.0		
S	BS-t	MT20	5.0	8.0		
T	BMWW+t	MT20	4.0	6.0		
U	BMW+w	MT20	3.0	10.0		
V	BMVW1-t	MT20	6.0	12.0	3.00	4.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 499.5 lbs FACTORED DOWN AT 28-0-6 ON TOP CHORD, AND 2553.9 lbs FACTORED DOWN AT 31-1-8, AND 71.7 lbs FACTORED DOWN AT 32-10-4, AND 71.7 lbs FACTORED DOWN AT 34-10-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DWG NO. TAM 450617
STRUCTURAL
COMPONENT ONLY

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	2639	0	2639	0	0	5-8	5-8
M	4803	0	4803	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	2130	1242 / 0	445 / 0	0 / 0	0 / 0	443 / 0	0 / 0
M	3865	2273 / 0	796 / 0	0 / 0	0 / 0	797 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.04 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF J-M.
2x4 DRY SPF No.2 T-BRACE AT C-V

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 26	-84.3	-84.3	0.12 (1)	10.00
B- C	0 / 16	-84.3	-84.3	0.30 (1)	10.00
C- D	-3944 / 0	-84.3	-84.3	0.52 (1)	3.01
D- E	-5242 / 0	-84.3	-84.3	0.46 (1)	3.43
E- F	-5242 / 0	-84.3	-84.3	0.37 (1)	3.50
F- G	-5696 / 0	-84.3	-84.3	0.43 (1)	3.24
G- H	-5697 / 0	-84.3	-84.3	0.55 (1)	3.17
H- I	-5697 / 0	-84.3	-84.3	0.55 (1)	3.17
I- J	-6166 / 0	-84.3	-84.3	0.77 (1)	2.04
J- K	0 / 13	-84.3	-84.3	0.32 (1)	10.00
K- L	0 / 26	-84.3	-84.3	0.12 (1)	10.00
V- B	-297 / 0	0.0	0.0	0.02 (1)	7.81
M- K	-305 / 0	0.0	0.0	0.02 (1)	7.81
V- U	0 / 3475	-28.0	-28.0	0.24 (1)	10.00
U- T	0 / 3475	-28.0	-28.0	0.23 (1)	10.00
T- S	0 / 3514	-28.0	-28.0	0.23 (1)	10.00
S- R	0 / 3514	-28.0	-28.0	0.23 (1)	10.00
R- Q	0 / 5593	-28.0	-28.0	0.34 (1)	10.00
Q- P	0 / 5472	-28.0	-28.0	0.33 (1)	10.00
P- O	0 / 5472	-28.0	-28.0	0.33 (1)	10.00
O- N	0 / 6678	-28.0	-28.0	0.46 (1)	10.00
N- W	0 / 6679	-28.0	-28.0	0.48 (1)	10.00
W- X	0 / 6679	-28.0	-28.0	0.48 (1)	10.00
X- M	0 / 6679	-28.0	-28.0	0.48 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
U- C	0 / 169	0.03 (3)
C- T	0 / 67	0.01 (2)
T- D	0 / 242	0.04 (2)
D- R	0 / 2146	0.38 (1)
R- E	-578 / 0	0.16 (1)
Q- G	-595 / 0	0.16 (1)
Q- I	0 / 529	0.09 (1)
O- I	0 / 1601	0.28 (1)
O- J	-1763 / 0	0.42 (1)
N- J	0 / 2533	0.45 (1)
V- C	-4142 / 0	0.64 (1)
J- M	-7951 / 0	0.93 (1)
R- F	-633 / 0	0.26 (1)
F- Q	0 / 550	0.10 (4)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
I	28-0-6	-45	-50	—	FRONT	VERT	DEAD
I	28-0-6	-455	-455	—	FRONT	VERT	SNOW

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.21")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.28")
ALLOWABLE DEFL.(TL)= L/360 (1.21")
CALCULATED VERT. DEFL.(TL) = L/ 996 (0.44")

CSI: TC=0.77 (I-J:1) , BC=0.48 (M-N:1) , WB=0.93 (J-M:1) , SSI=0.23 (G-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (M) (INPUT = 0.90)
JSI METAL= 1.00 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285876	T620Z	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
N	31-1-8	-2554	-2554	---	FRONT	VERT	TOTAL
W	32-10-4	-41	-72	---	FRONT	VERT	TOTAL
X	34-10-4	-41	-72	---	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
9777
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 45096-17 P6 of 6

STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285876	T630	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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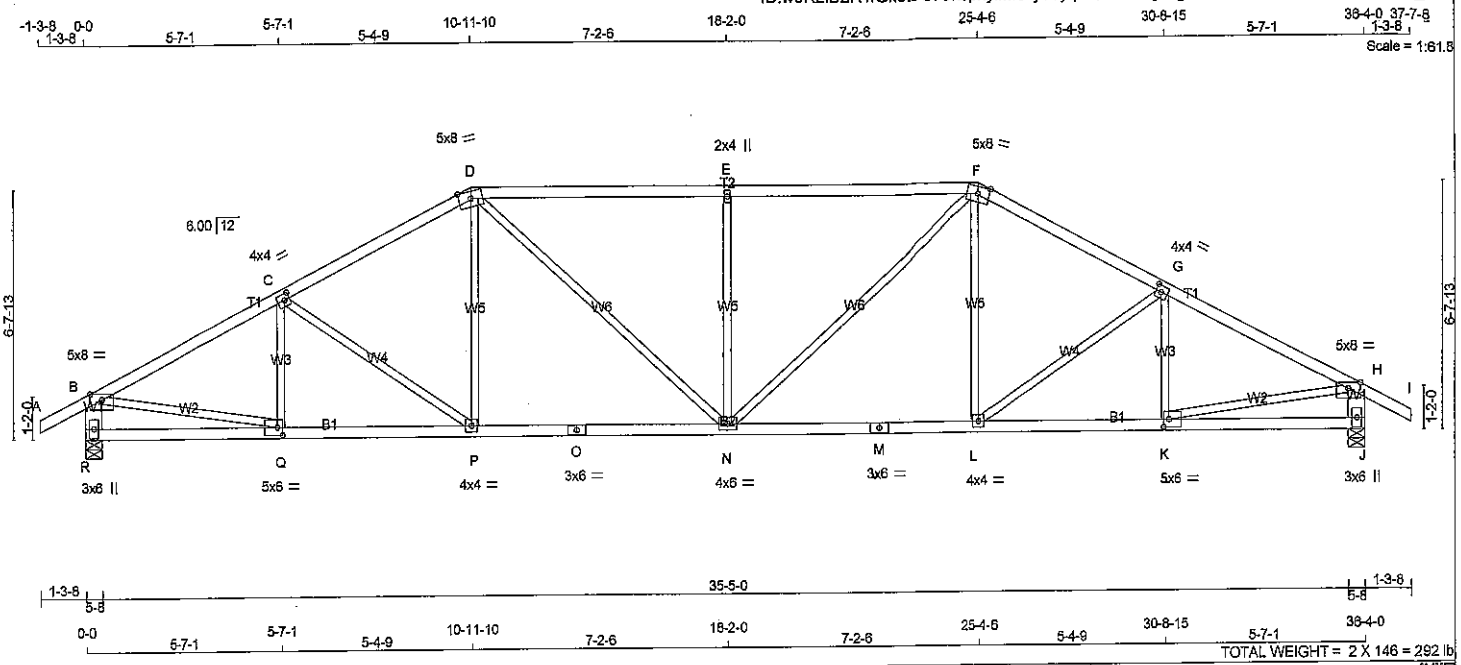

TOTAL WEIGHT = 2 X 146 = 292 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
J - H	2x8	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	8.0	Edge	
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	8.0	Edge	
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
H	TMVW-p	MT20	5.0	8.0	Edge	
J	BMV1+p	MT20	3.0	6.0		
K	BMWW-t	MT20	5.0	6.0	2.50	1.75
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	6.0		
O	BS-t	MT20	3.0	6.0		
P	BMWW-t	MT20	4.0	4.0		
Q	BMWW-t	MT20	5.0	6.0	2.50	1.75
R	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2154	0	2154	0	0	5-8	5-8
J	2154	0	2154	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1752	999 / 0	382 / 0	0 / 0	0 / 0	371 / 0	0 / 0
J	1752	999 / 0	382 / 0	0 / 0	0 / 0	371 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.22 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 26	-84.3	-84.3	0.11 (1)	10.00	
B- C	-3033 / 0	-84.3	-84.3	0.48 (1)	3.63	
C- D	-2787 / 0	-84.3	-84.3	0.44 (1)	3.80	
D- E	-3013 / 0	-84.3	-84.3	0.78 (1)	3.22	
E- F	-3013 / 0	-84.3	-84.3	0.78 (1)	3.22	
F- G	-2787 / 0	-84.3	-84.3	0.44 (1)	3.80	
G- H	-3033 / 0	-84.3	-84.3	0.48 (1)	3.63	
H- I	0 / 26	-84.3	-84.3	0.11 (1)	10.00	
R- B	-2085 / 0	0.0	0.0	0.13 (1)	7.03	
J- H	-2085 / 0	0.0	0.0	0.13 (1)	7.03	
R- Q	0 / 0	-28.0	-28.0	0.18 (2)	10.00	
Q- P	0 / 2733	-28.0	-28.0	0.57 (1)	10.00	
P- O	0 / 2475	-28.0	-28.0	0.59 (2)	10.00	
O- N	0 / 2475	-28.0	-28.0	0.59 (2)	10.00	
N- M	0 / 2475	-28.0	-28.0	0.59 (2)	10.00	
M- L	0 / 2475	-28.0	-28.0	0.59 (2)	10.00	
L- K	0 / 2733	-28.0	-28.0	0.57 (1)	10.00	
K- J	0 / 0	-28.0	-28.0	0.18 (2)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-263 / 80	0.06 (1)
C- P	-317 / 0	0.25 (1)
P- D	0 / 422	0.10 (2)
D- N	0 / 718	0.16 (1)
N- E	-746 / 0	0.55 (1)
N- F	0 / 718	0.16 (1)
L- F	0 / 422	0.10 (2)
L- G	-317 / 0	0.25 (1)
K- G	-263 / 80	0.06 (1)
B- O	0 / 2765	0.62 (1)
K- H	0 / 2765	0.62 (1)

DWG NO. TAM 45057-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.21")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.22")
ALLOWABLE DEFL.(TL)= L/360 (1.21")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.36")

CSI: TC=0.78 (D-E:1) , BC=0.59 (N-P:2) , WB=0.62 (B-Q:1) , SSI=0.29 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (N) (INPUT = 0.90)
JSI METAL= 0.76 (M) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285876	T640	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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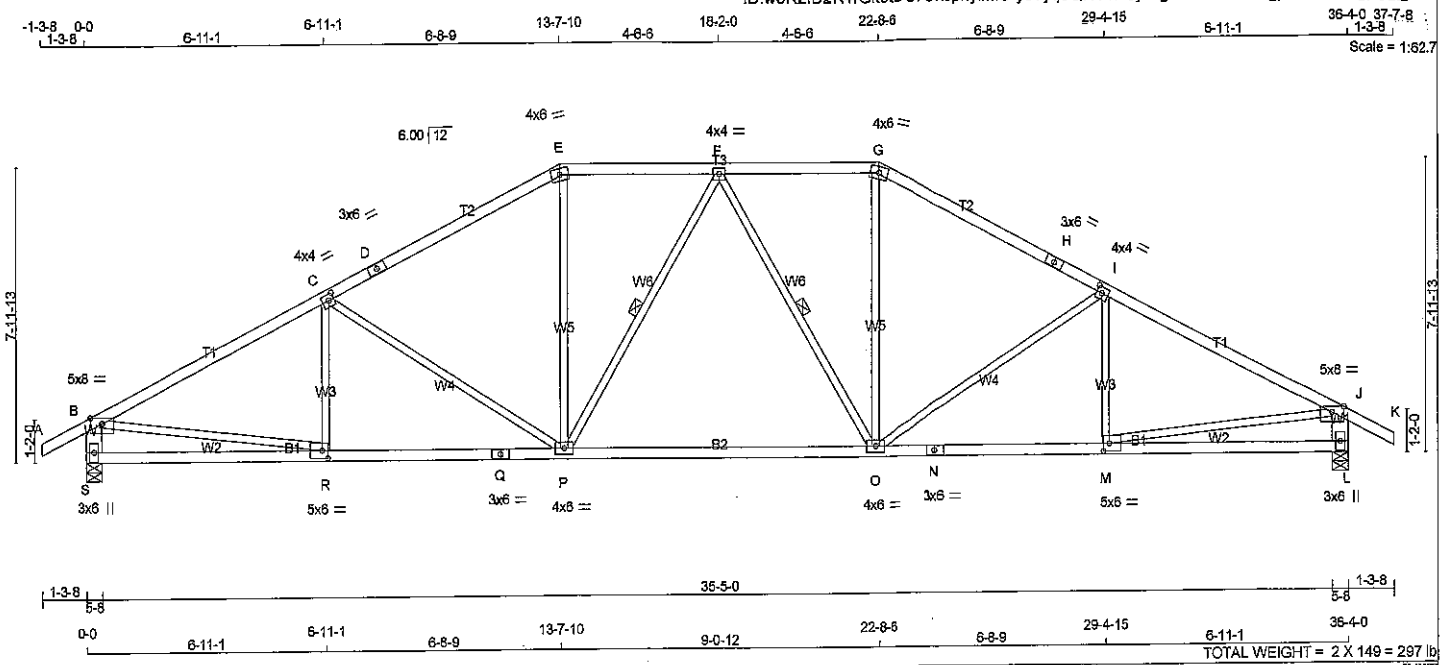


[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	6.0		
E	TTW-m	MT20	4.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTW-m	MT20	4.0	6.0		
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	4.0	4.0	2.00	1.75
J	TMVW-p	MT20	5.0	8.0	Edge	
L	BMV1+p	MT20	3.0	6.0		
M	BMWW-t	MT20	5.0	6.0	2.50	2.00
N	BS-t	MT20	3.0	6.0		
O	BMWWW-t	MT20	4.0	6.0		
P	BMWWW-t	MT20	4.0	6.0		
Q	BS-t	MT20	3.0	6.0		
R	BMWW-t	MT20	5.0	6.0	2.50	2.00
S	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	2154	0	2154	0	0	5-8	5-8
L	2154	0	2154	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1752	999 / 0	382 / 0	0 / 0	0 / 0	371 / 0	0 / 0
L	1752	999 / 0	382 / 0	0 / 0	0 / 0	371 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.28 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-P, F-O.

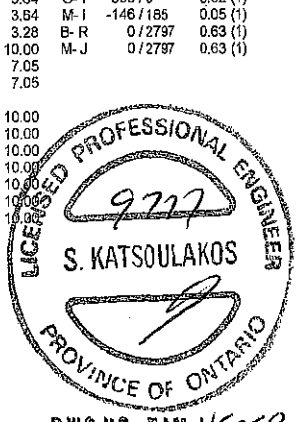
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 26	-84.3	-84.3	0.11 (1)	10.00
B- C	-3074 / 0	-84.3	-84.3	0.76 (1)	3.28
C- D	-2576 / 0	-84.3	-84.3	0.67 (1)	3.64
D- E	-2576 / 0	-84.3	-84.3	0.67 (1)	3.64
E- F	-2285 / 0	-84.3	-84.3	0.27 (1)	4.29
F- G	-2285 / 0	-84.3	-84.3	0.27 (1)	4.29
G- H	-2576 / 0	-84.3	-84.3	0.67 (1)	3.64
H- I	-2576 / 0	-84.3	-84.3	0.67 (1)	3.64
I- J	-3074 / 0	-84.3	-84.3	0.76 (1)	3.28
J- K	0 / 26	-84.3	-84.3	0.11 (1)	10.00
S- B	-2071 / 0	0.0	0.0	0.13 (1)	7.05
L- J	-2071 / 0	0.0	0.0	0.13 (1)	7.05
S- R	0 / 0	-28.0	-28.0	0.28 (3)	10.00
R- Q	0 / 2776	-28.0	-28.0	0.69 (2)	10.00
Q- P	0 / 2776	-28.0	-28.0	0.69 (2)	10.00
P- O	0 / 2418	-28.0	-28.0	0.65 (2)	10.00
O- N	0 / 2776	-28.0	-28.0	0.69 (2)	10.00
N- M	0 / 2776	-28.0	-28.0	0.69 (2)	10.00
M- L	0 / 0	-28.0	-28.0	0.28 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R- C	-146 / 185	0.05 (1)
C- P	-593 / 0	0.82 (1)
P- E	0 / 775	0.17 (1)
P- F	-265 / 0	0.15 (1)
F- O	-265 / 0	0.15 (1)
O- G	0 / 775	0.17 (1)
O- I	-593 / 0	0.82 (1)
M- I	-146 / 185	0.05 (1)
B- R	0 / 2797	0.63 (1)
M- J	0 / 2797	0.63 (1)



DWG NO. TAM 45058-17

STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.21")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.27")
ALLOWABLE DEFL.(TL)= L/360 (1.21")
CALCULATED VERT. DEFL.(TL) = L/ 985 (0.44")

CSI: TC=0.78 (B-C:1) , BC=0.69 (P-R:2) , WB=0.82 (C-P:1) , SSI=0.25 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (B) (INPUT = 0.90)
JSI METAL= 0.71 (N) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285876	T650	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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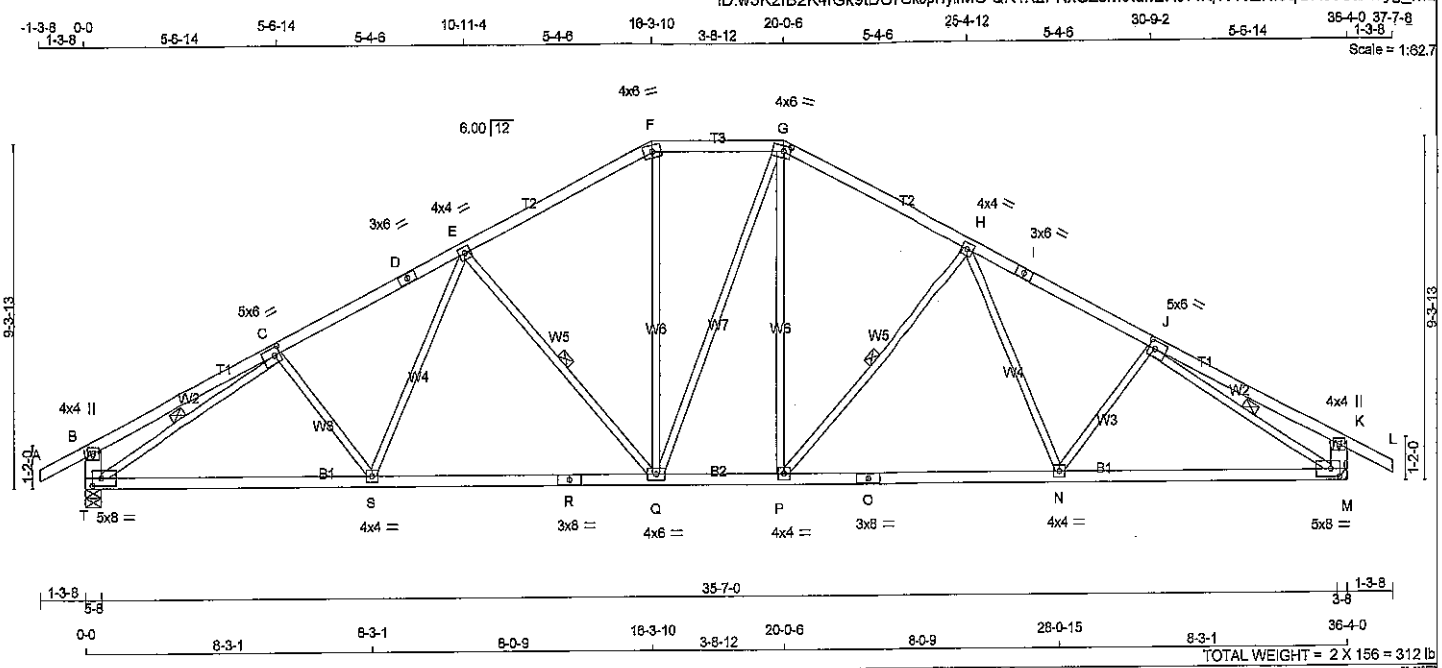


[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
T - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.00
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	6.0		
G	TTWW-m	MT20	4.0	8.0	1.75	2.25
H	TMWW-t	MT20	4.0	4.0		
I	TS-t	MT20	3.0	6.0		
J	TMWW-t	MT20	5.0	6.0	2.50	2.00
K	TMV+p	MT20	4.0	4.0		
M	BMVW1-t	MT20	5.0	8.0	2.50	3.00
N, P, S						
N	BMWW-t	MT20	4.0	4.0		
O	BS-t	MT20	3.0	8.0		
Q	BMWWW-t	MT20	4.0	6.0		
R	BS-t	MT20	3.0	8.0		
T	BMVW1-t	MT20	5.0	8.0	2.50	3.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
T	2154	0	2154	0	0	IN-SX	IN-SX
M	2154	0	2154	0	0	5-8	5-8
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1752	999 / 0	382 / 0	0 / 0	0 / 0	371 / 0	0 / 0
M	1752	999 / 0	382 / 0	0 / 0	0 / 0	371 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.76 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

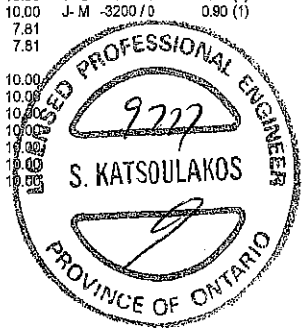
1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-Q, H-P, C-T, J-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 26	-84.3	-84.3	0.11 (1)	10.00	C- S	-72 / 148	0.03 (3)	
B- C	0 / 18	-84.3	-84.3	0.30 (1)	10.00	S- E	0 / 424	0.10 (2)	
C- D	-2939 / 0	-84.3	-84.3	0.41 (1)	3.76	E- Q	-701 / 0	0.34 (1)	
D- E	-2939 / 0	-84.3	-84.3	0.41 (1)	3.76	Q- F	0 / 663	0.15 (1)	
E- F	-2298 / 0	-84.3	-84.3	0.37 (1)	4.20	Q- G	0 / 4	0.00 (1)	
F- G	-2044 / 0	-84.3	-84.3	0.20 (1)	4.57	P- G	0 / 679	0.15 (1)	
G- H	-2296 / 0	-84.3	-84.3	0.37 (1)	4.20	P- H	-703 / 0	0.34 (1)	
H- I	-2940 / 0	-84.3	-84.3	0.41 (1)	3.76	H- N	0 / 426	0.10 (2)	
I- J	-2940 / 0	-84.3	-84.3	0.41 (1)	3.76	N- J	-71 / 148	0.03 (3)	
J- K	0 / 18	-84.3	-84.3	0.30 (1)	10.00	T- C	-3199 / 0	0.90 (1)	
K- L	0 / 26	-84.3	-84.3	0.11 (1)	10.00	J- M	-3200 / 0	0.90 (1)	
T- B	-305 / 0	0.0	0.0	0.02 (1)	7.81				
M- K	-305 / 0	0.0	0.0	0.02 (1)	7.81				
T- S	0 / 2674	-28.0	-28.0	0.79 (2)	10.00				
S- R	0 / 2494	-28.0	-28.0	0.77 (2)	10.00				
R- Q	0 / 2494	-28.0	-28.0	0.77 (2)	10.00				
Q- P	0 / 2042	-28.0	-28.0	0.46 (2)	10.00				
P- O	0 / 2494	-28.0	-28.0	0.78 (2)	10.00				
O- N	0 / 2494	-28.0	-28.0	0.78 (2)	10.00				
N- M	0 / 2674	-28.0	-28.0	0.79 (2)	10.00				



DWG NO. TAM45059-17

STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF	
	DL =	3.0	PSF	
BOT CH.	LL =	10.5	PSF	
	DL =	7.0	PSF	
TOTAL LOAD	=	46.1	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.21")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.22")
ALLOWABLE DEFL.(TL)= L/360 (1.21")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.38")

CSI: TC=0.41 (H-J:1) , BC=0.79 (M-N:2) , WB=0.90 (J-M:1) , SSI=0.19 (M-N:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (M) (INPUT = 0.90)
JSI METAL= 0.80 (O) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285876	T660	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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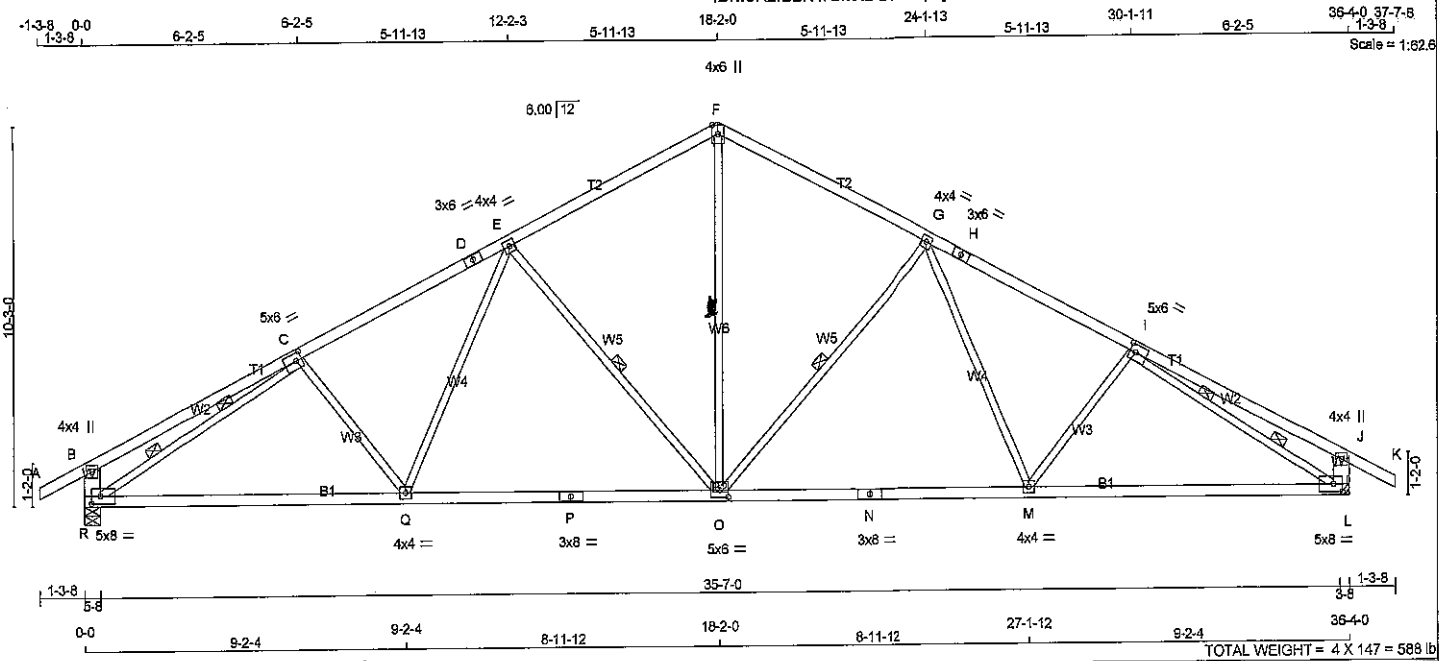

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	5.0	8.0	2.50	2.00
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTW+p	MT20	4.0	6.0	Edge	
G	TMWW-t	MT20	4.0	4.0		
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	5.0	6.0	2.50	2.00
J	TMV+p	MT20	4.0	4.0		
L	BMVW1-t	MT20	5.0	8.0	2.50	3.00
M	BMWW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	8.0		
O	BMWWW-t	MT20	5.0	6.0	2.25	3.00
P	BS-t	MT20	3.0	8.0		
Q	BMWW-t	MT20	4.0	4.0		
R	BMVW1-t	MT20	5.0	8.0	2.50	3.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2154	0	2154	0	0	5-8	5-8
L	2154	0	2154	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1752	999 / 0	352 / 0	0 / 0	0 / 0	371 / 0	0 / 0
L	1752	999 / 0	352 / 0	0 / 0	0 / 0	371 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.70 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-O, E-O. F-O
2 LATERAL BRACE(S) AT 1/ 3 LENGTH OF C-R, I-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 26	-84.3	-84.3	0.11 (1)	10.00
B- C	0 / 21	-84.3	-84.3	0.39 (1)	10.00
C- D	-2896 / 0	-84.3	-84.3	0.50 (1)	3.70
D- E	-2896 / 0	-84.3	-84.3	0.50 (1)	3.70
E- F	-2124 / 0	-84.3	-84.3	0.45 (1)	4.25
F- G	-2124 / 0	-84.3	-84.3	0.45 (1)	4.25
G- H	-2896 / 0	-84.3	-84.3	0.50 (1)	3.70
H- I	-2896 / 0	-84.3	-84.3	0.50 (1)	3.70
I- J	0 / 21	-84.3	-84.3	0.39 (1)	10.00
J- K	0 / 26	-84.3	-84.3	0.11 (1)	10.00
R- B	-325 / 0	0.0	0.0	0.02 (1)	7.81
L- J	-325 / 0	0.0	0.0	0.02 (1)	7.81
R- Q	0 / 2692	-28.0	-28.0	0.88 (2)	10.00
Q- P	0 / 2413	-28.0	-28.0	0.85 (2)	10.00
P- O	0 / 2413	-28.0	-28.0	0.85 (2)	10.00
O- N	0 / 2413	-28.0	-28.0	0.85 (2)	10.00
N- M	0 / 2413	-28.0	-28.0	0.85 (2)	10.00
M- L	0 / 2692	-28.0	-28.0	0.88 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- F	0 / 1475	0.33 (1)
O- G	-816 / 0	0.48 (1)
G- M	0 / 509	0.11 (2)
M- I	-163 / 124	0.07 (1)
E- O	-816 / 0	0.48 (1)
Q- E	0 / 509	0.11 (2)
C- Q	-163 / 124	0.07 (1)
R- C	-3197 / 0	1.00 (1)
I- L	-3197 / 0	1.00 (1)

DWG NO. TAM4506917
STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.21")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.31")
ALLOWABLE DEFL.(TL)= L/360 (1.21")
CALCULATED VERT. DEFL.(TL) = L/ 853 (0.51")

CSI: TC=0.50 (G-I:1) , BC=0.88 (Q-R:2) , WB=1.00 (I-L:1) , SSI=0.21 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (L) (INPUT = 0.90)
JSI METAL= 0.80 (N) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285876	T670S	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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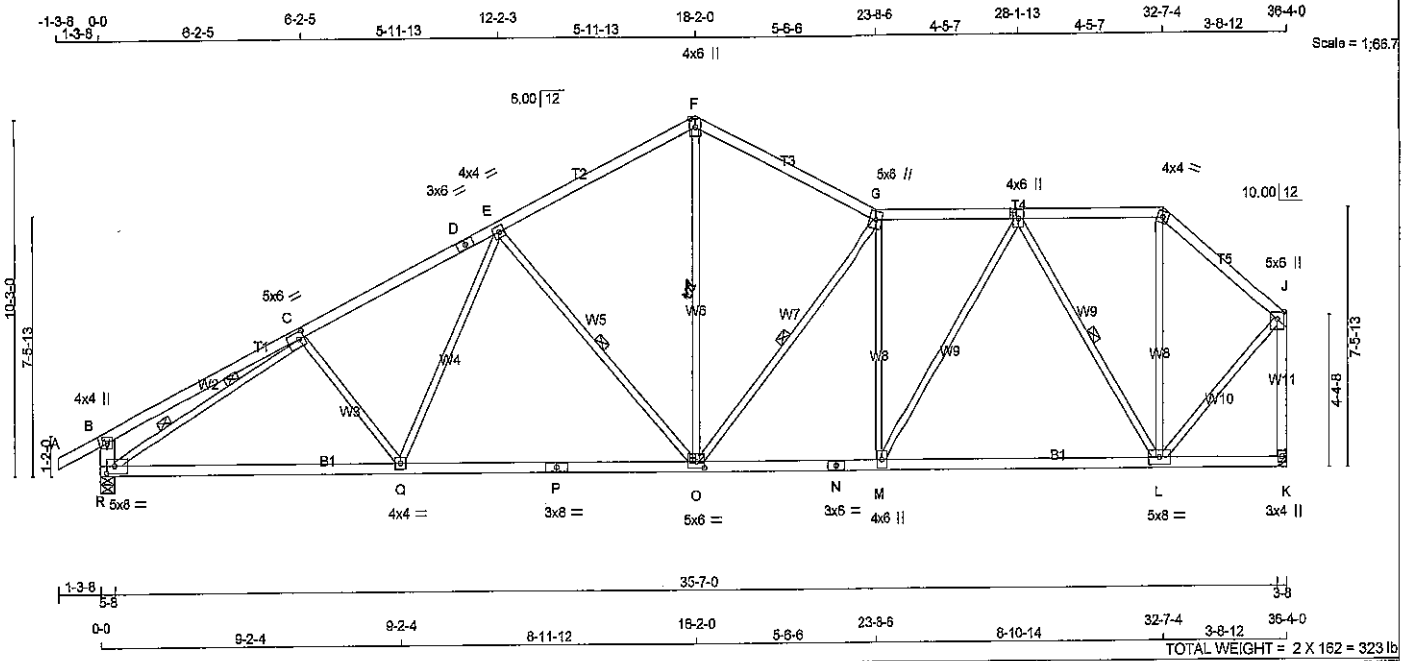

TOTAL WEIGHT = 2 X 162 = 323 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - J	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.00
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTW+p	MT20	4.0	6.0	Edge	
G	TTWW+m	MT20	5.0	6.0		
H	TMWW+t	MT20	4.0	6.0		
I	TTW-m	MT20	4.0	4.0		
J	TMVW+p	MT20	5.0	6.0	Edge	
K	BMV1+p	MT20	3.0	4.0		
L	BMWWW-t	MT20	5.0	6.0		
M	BMWW+t	MT20	4.0	6.0		
N	BS-t	MT20	3.0	6.0		
O	BMWWW-t	MT20	5.0	6.0	2.25	3.00
P	BS-t	MT20	3.0	6.0		
Q	BMWW-t	MT20	4.0	4.0		
R	BMVW1-t	MT20	5.0	6.0	2.50	3.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
R	2154	0	2154	0	0	IN-SX	IN-SX
K	2039	0	2039	0	0	5-8	5-8
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1752	999 / 0	382 / 0	0 / 0	0 / 0	371 / 0	0 / 0
K	1674	930 / 0	382 / 0	0 / 0	0 / 0	363 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.70 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-O, G-O, H-L.
2 LATERAL BRACE(S) AT 1/ 3 LENGTH OF C-R.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 26	-84.3	-84.3	0.11 (1)	10.00	C- Q	-163 / 124	0.07 (1)
B- C	0 / 21	-84.3	-84.3	0.39 (1)	10.00	Q- E	0 / 524	0.12 (2)
C- D	-2898 / 0	-84.3	-84.3	0.50 (1)	3.70	E- O	-822 / 0	0.49 (1)
D- E	-2898 / 0	-84.3	-84.3	0.50 (1)	3.70	O- F	0 / 1436	0.32 (1)
E- F	-2118 / 0	-84.3	-84.3	0.44 (1)	4.25	O- G	-788 / 0	0.45 (1)
F- G	-2095 / 0	-84.3	-84.3	0.41 (1)	4.30	M- G	-680 / 0	0.70 (1)
G- H	-2345 / 0	-84.3	-84.3	0.28 (1)	4.24	M- H	0 / 1039	0.23 (1)
H- I	-994 / 0	-84.3	-84.3	0.22 (1)	6.00	H- L	-1572 / 0	0.76 (1)
I- J	-1288 / 0	-84.3	-84.3	0.17 (1)	5.51	L- I	0 / 523	0.12 (1)
R- B	-325 / 0	0.0	0.0	0.02 (1)	7.81	R- C	-3200 / 0	1.00 (1)
K- J	-2025 / 0	0.0	0.0	0.66 (1)	5.94	L- J	0 / 1433	0.32 (1)
R- Q	0 / 2694	-28.0	-28.0	0.91 (2)	10.00			
Q- P	0 / 2411	-28.0	-28.0	0.88 (2)	10.00			
P- O	0 / 2411	-28.0	-28.0	0.88 (2)	10.00			
O- N	0 / 2355	-28.0	-28.0	0.59 (2)	10.00			
N- M	0 / 2355	-28.0	-28.0	0.59 (2)	10.00			
M- L	0 / 1807	-28.0	-28.0	0.57 (2)	10.00			
L- K	0 / 0	-28.0	-28.0	0.36 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.8	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.21")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.29")
ALLOWABLE DEFL.(TL)= L/360 (1.21")
CALCULATED VERT. DEFL.(TL) = L/ 907 (0.48")

CSI: TC=0.66 (J-K:1) , BC=0.91 (Q-R:2) , WB=1.00 (C-R:1) , SSI=0.21 (Q-R:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (R) (INPUT = 0.90)
JSI METAL= 0.85 (P) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 45061-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285876	T680S	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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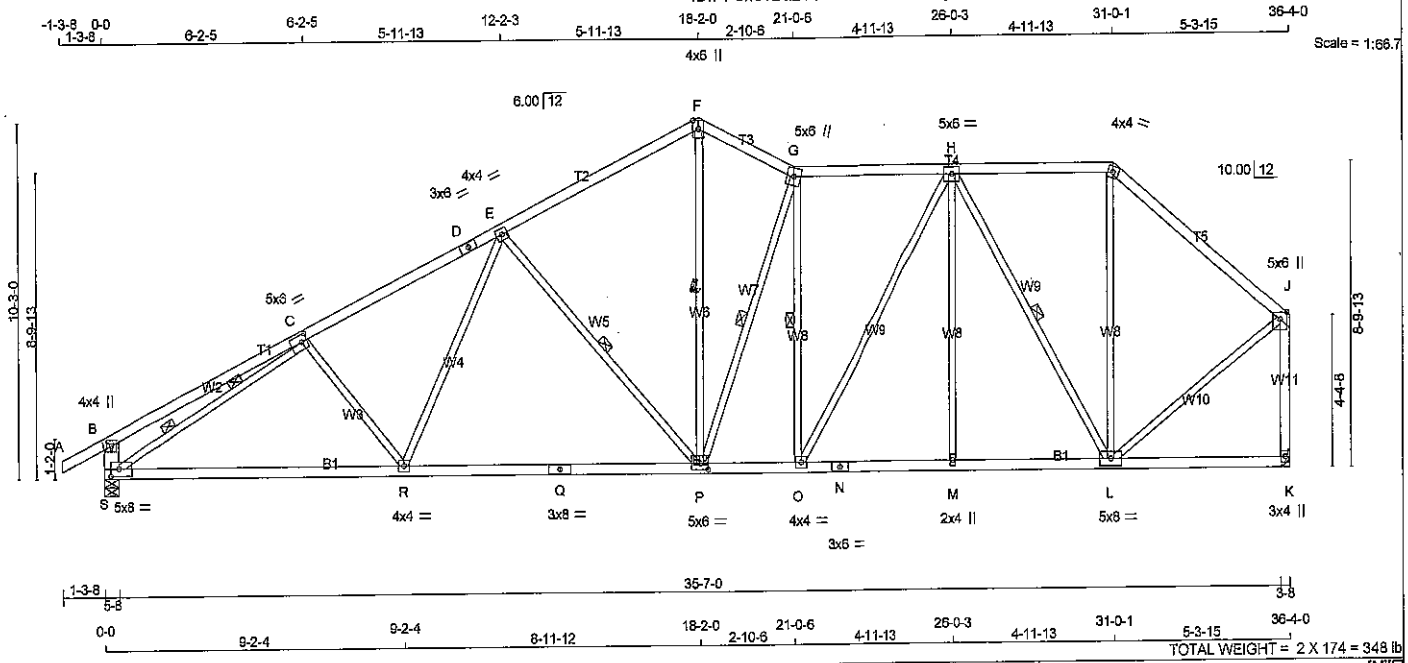

TOTAL WEIGHT = 2 X 174 = 348 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - J	2x4	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.00
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTW+p	MT20	4.0	6.0	Edge	
G	TTWW+m	MT20	5.0	6.0		
H	TMWWW-t	MT20	5.0	6.0		
I	TTW-m	MT20	4.0	4.0		
J	TMVW+p	MT20	5.0	6.0	Edge	
K	BMV1+p	MT20	3.0	4.0		
L	BMWWW-t	MT20	5.0	8.0		
M	BMW+w	MT20	2.0	4.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWWW-t	MT20	5.0	6.0	2.25	3.00
Q	BS-t	MT20	3.0	8.0		
R	BMWW-t	MT20	4.0	4.0		
S	BMVW1-t	MT20	5.0	8.0	2.50	3.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	2154	0	2154	0	0	5-8	5-8
K	2039	0	2039	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1752	999 / 0	382 / 0	0 / 0	0 / 0	371 / 0	0 / 0
K	1674	930 / 0	382 / 0	0 / 0	0 / 0	363 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.70 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-P, G-P, G-O, H-L. P-P

2 LATERAL BRACE(S) AT 1/ 3 LENGTH OF C-S.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 26	-84.3	-84.3	0.11 (1)	10.00
B- C	0 / 21	-84.3	-84.3	0.39 (1)	10.00
C- D	-2898 / 0	-84.3	-84.3	0.50 (1)	3.70
D- E	-2898 / 0	-84.3	-84.3	0.50 (1)	3.70
E- F	-2119 / 0	-84.3	-84.3	0.44 (1)	4.25
F- G	-2095 / 0	-84.3	-84.3	0.14 (1)	4.59
G- H	-2116 / 0	-84.3	-84.3	0.33 (1)	4.36
H- I	-1115 / 0	-84.3	-84.3	0.28 (1)	5.67
I- J	-1446 / 0	-84.3	-84.3	0.35 (1)	5.06
S- B	-325 / 0	0.0	0.0	0.02 (1)	7.81
K- J	-1978 / 0	0.0	0.0	0.65 (1)	6.00
S- R	0 / 2894	-28.0	-28.0	0.90 (2)	10.00
R- Q	0 / 2412	-28.0	-28.0	0.87 (2)	10.00
Q- P	0 / 2412	-28.0	-28.0	0.87 (2)	10.00
P- O	0 / 2122	-28.0	-28.0	0.54 (2)	10.00
O- N	0 / 1802	-28.0	-28.0	0.36 (1)	10.00
N- M	0 / 1802	-28.0	-28.0	0.36 (1)	10.00
M- L	0 / 1802	-28.0	-28.0	0.40 (2)	10.00
L- K	0 / 0	-28.0	-28.0	0.19 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- R	-183 / 124	0.07 (1)
R- E	0 / 521	0.12 (2)
E- P	-821 / 0	0.49 (1)
P- F	0 / 1547	0.35 (1)
P- G	-772 / 0	0.43 (1)
O- G	-484 / 0	0.24 (1)
O- H	0 / 631	0.14 (1)
M- H	0 / 242	0.05 (2)
H- L	-1382 / 0	0.95 (1)
L- I	0 / 540	0.12 (1)
S- C	-3199 / 0	1.00 (1)
L- J	0 / 1382	0.31 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.21")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.29")
ALLOWABLE DEFL.(TL)= L/360 (1.21")
CALCULATED VERT. DEFL.(TL) = L/ 904 (0.48")

CSI: TC=0.65 (J-K:1) , BC=0.90 (R-S:2) , WB=1.00 (C-S:1) , SSI=0.21 (R-S:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (S) (INPUT = 0.90)
JSI METAL= 0.84 (Q) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER 9211 S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM 45062-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285876	T690S	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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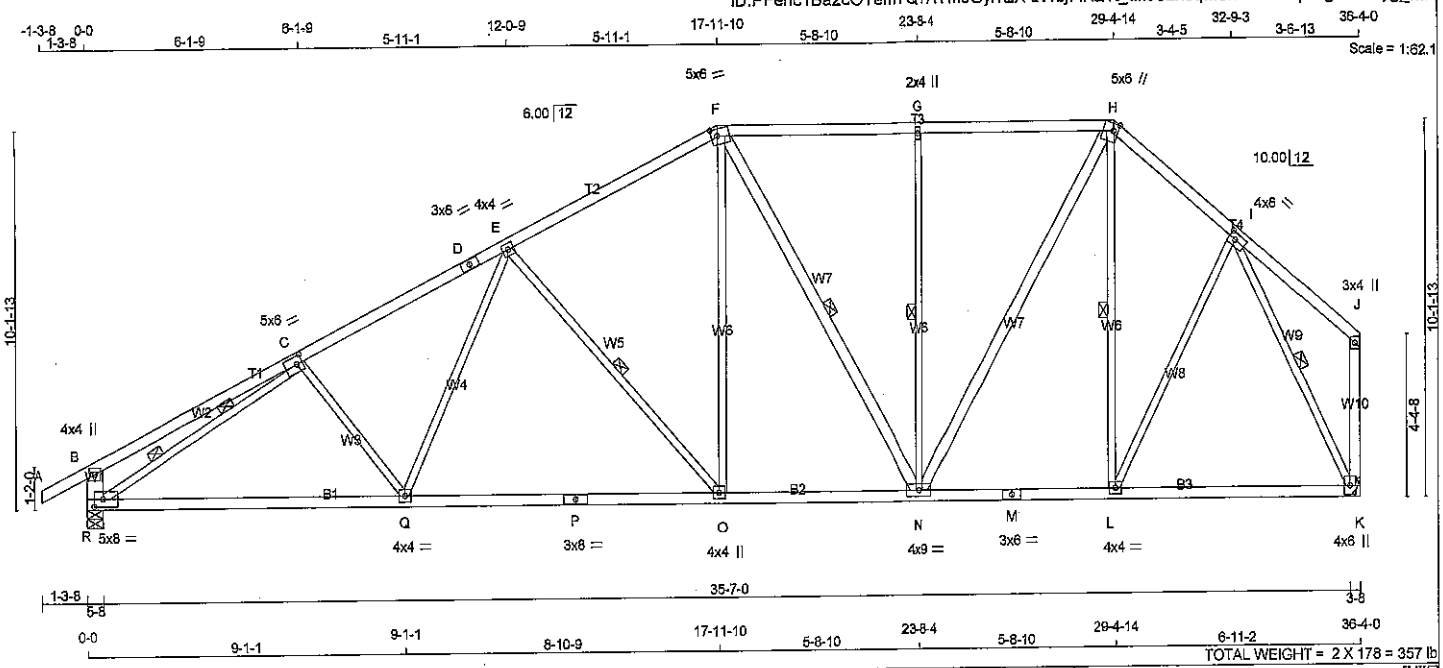

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
F - N	2x4	DRY	No.2	SPF
N - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.00
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMW+w	MT20	2.0	4.0		
H	TTWW+m	MT20	5.0	6.0	2.25	1.50
I	TMWW-t	MT20	4.0	6.0	2.00	2.25
J	TMV+p	MT20	3.0	4.0		
K	BMVW1+p	MT20	4.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	9.0		
O	BMWW+t	MT20	4.0	4.0		
P	BS-t	MT20	3.0	8.0		
Q	BMWW-t	MT20	4.0	4.0		
R	BMVW1-t	MT20	5.0	6.0	2.50	3.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
R	2154	0	2154	0	0	IN-SX	IN-SX
K	2039	0	2039	0	0	5-8	5-8
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1752	999 / 0	382 / 0	0 / 0	0 / 0	371 / 0	0 / 0
K	1674	930 / 0	382 / 0	0 / 0	0 / 0	363 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.71 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-O, F-N, G-N, H-L, I-K.
2 LATERAL BRACE(S) AT 1/ 3 LENGTH OF C-R.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 26	-84.3	-84.3	0.11 (1)	10.00	C- Q	-153 / 127	0.06 (1)
B- C	0 / 20	-84.3	-84.3	0.38 (1)	10.00	Q- E	0 / 511	0.11 (2)
C- D	-2903 / 0	-84.3	-84.3	0.50 (1)	3.71	E- O	-813 / 0	0.48 (1)
D- E	-2903 / 0	-84.3	-84.3	0.50 (1)	3.71	O- F	0 / 821	0.18 (1)
E- F	-2132 / 0	-84.3	-84.3	0.44 (1)	4.25	F- N	-301 / 0	0.21 (1)
F- G	-1743 / 0	-84.3	-84.3	0.40 (1)	4.63	N- G	-588 / 0	0.42 (1)
G- H	-1743 / 0	-84.3	-84.3	0.40 (1)	4.63	N- H	0 / 1126	0.18 (1)
H- I	-1562 / 0	-84.3	-84.3	0.15 (1)	5.15	L- H	-284 / 36	0.20 (1)
I- J	0 / 20	-84.3	-84.3	0.16 (1)	10.00	L- I	0 / 557	0.13 (1)
R- B	-323 / 0	0.0	0.0	0.02 (1)	7.81	R- C	-3200 / 0	0.98 (1)
K- J	-116 / 0	0.0	0.0	0.04 (1)	7.81	I- K	-2084 / 0	0.82 (1)
R- Q	0 / 2692	-28.0	-28.0	0.89 (2)	10.00			
Q- P	0 / 2421	-28.0	-28.0	0.86 (2)	10.00			
P- O	0 / 2421	-28.0	-28.0	0.86 (2)	10.00			
O- N	0 / 1894	-28.0	-28.0	0.53 (2)	10.00			
N- M	0 / 1177	-28.0	-28.0	0.46 (2)	10.00			
M- L	0 / 1177	-28.0	-28.0	0.46 (2)	10.00			
L- K	0 / 928	-28.0	-28.0	0.42 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.21")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.28")
ALLOWABLE DEFL.(TL)= L/360 (1.21")
CALCULATED VERT. DEFL.(TL) = L/ 950 (0.46")

CSI: TC=0.50 (C-E:1) , BC=0.89 (Q-R:2) , WB=0.98 (C-R:1) , SSI=0.23 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.84 (P) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
9777
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM-45063-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43854	DRWG NO.
285876	T700	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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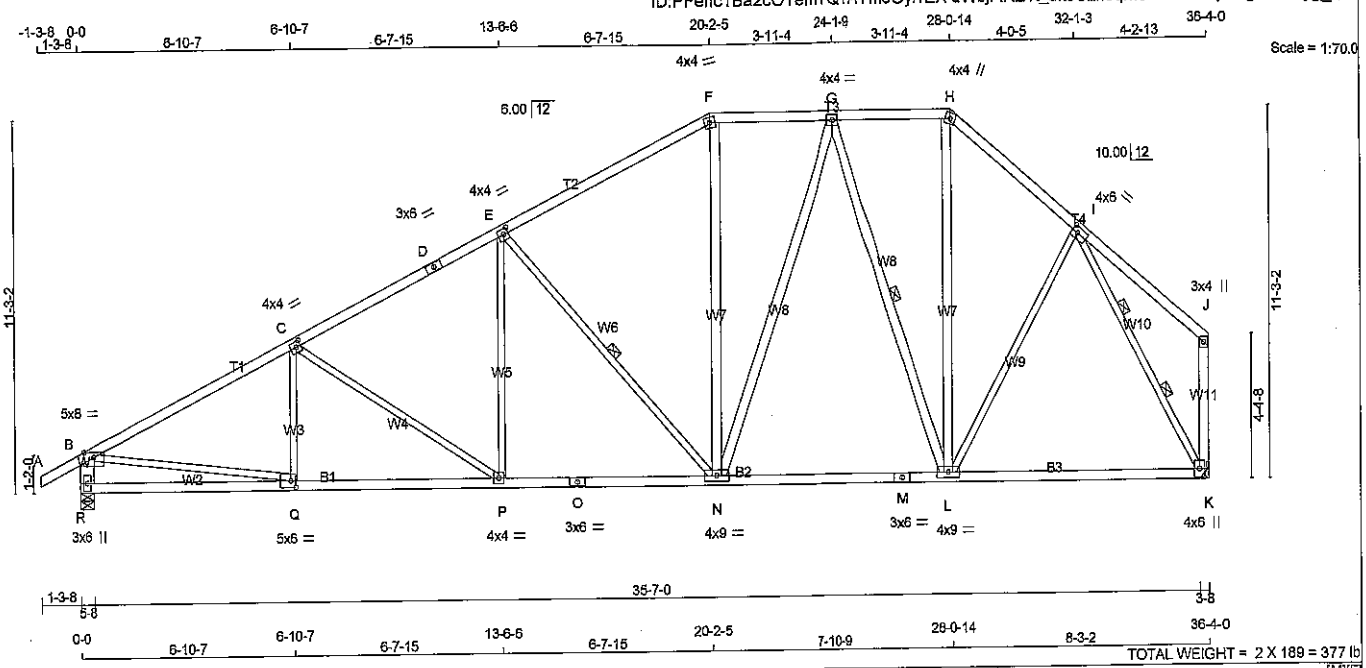

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF
N - G	2x4	DRY	No.2	SPF
G - L	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.75
F	TTW-m	MT20	4.0	4.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTW+m	MT20	4.0	4.0		
I	TMWW-t	MT20	4.0	6.0	2.00	2.25
J	TMV+p	MT20	3.0	4.0		
K	BMVW1+p	MT20	4.0	6.0		
L	BMWWW-t	MT20	4.0	9.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	9.0		
O	BS-t	MT20	3.0	6.0		
P	BMWW-t	MT20	4.0	4.0		
Q	BMWW-t	MT20	5.0	6.0	2.50	2.00
R	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
R	2154	0	2154	0	0	IN-SX	IN-SX
K	2039	0	2039	0	0	5-8	5-8
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1752	999 / 0	382 / 0	0 / 0	0 / 0	371 / 0	0 / 0
K	1674	930 / 0	382 / 0	0 / 0	0 / 0	363 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.37 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-N, G-L.
2 LATERAL BRACE(S) AT 1/ 3 LENGTH OF I-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 26	-84.3	-84.3	0.11 (1)	10.00	Q- C	-137 / 199	0.04 (3)
B- C	-3059 / 0	-84.3	-84.3	0.75 (1)	3.37	C- P	-508 / 0	0.66 (1)
C- D	-2607 / 0	-84.3	-84.3	0.57 (1)	3.80	P- E	0 / 523	0.12 (2)
D- E	-2607 / 0	-84.3	-84.3	0.57 (1)	3.80	E- N	-998 / 0	0.76 (1)
E- F	-1890 / 0	-84.3	-84.3	0.53 (1)	4.33	N- F	0 / 478	0.08 (1)
F- G	-1674 / 0	-84.3	-84.3	0.19 (1)	4.96	N- G	0 / 485	0.06 (1)
G- H	-1220 / 0	-84.3	-84.3	0.17 (1)	5.82	G- L	-904 / 0	0.69 (1)
H- I	-1604 / 0	-84.3	-84.3	0.21 (1)	5.04	L- H	0 / 746	0.12 (1)
I- J	0 / 25	-84.3	-84.3	0.22 (1)	10.00	L- I	0 / 422	0.09 (1)
R- B	-2073 / 0	0.0	0.0	0.13 (1)	7.04	B- Q	0 / 2778	0.63 (1)
K- J	-137 / 0	0.0	0.0	0.04 (1)	7.81	I- K	-2067 / 0	0.93 (1)
R- Q	0 / 0	-28.0	-28.0	0.30 (3)	10.00			
Q- P	0 / 2757	-28.0	-28.0	0.62 (2)	10.00			
P- O	0 / 2332	-28.0	-28.0	0.53 (2)	10.00			
O- N	0 / 2332	-28.0	-28.0	0.53 (2)	10.00			
N- M	0 / 1515	-28.0	-28.0	0.65 (2)	10.00			
M- L	0 / 1515	-28.0	-28.0	0.65 (2)	10.00			
L- K	0 / 1002	-28.0	-28.0	0.58 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.21")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.20")
ALLOWABLE DEFL.(TL)= L/360 (1.21")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.33")

CSI: TC=0.75 (B-C:1) , BC=0.65 (L-N:2) , WB=0.93 (I-K:1) , SSI=0.24 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.89 (O) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
9772
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. T&M 45064-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285492	T720	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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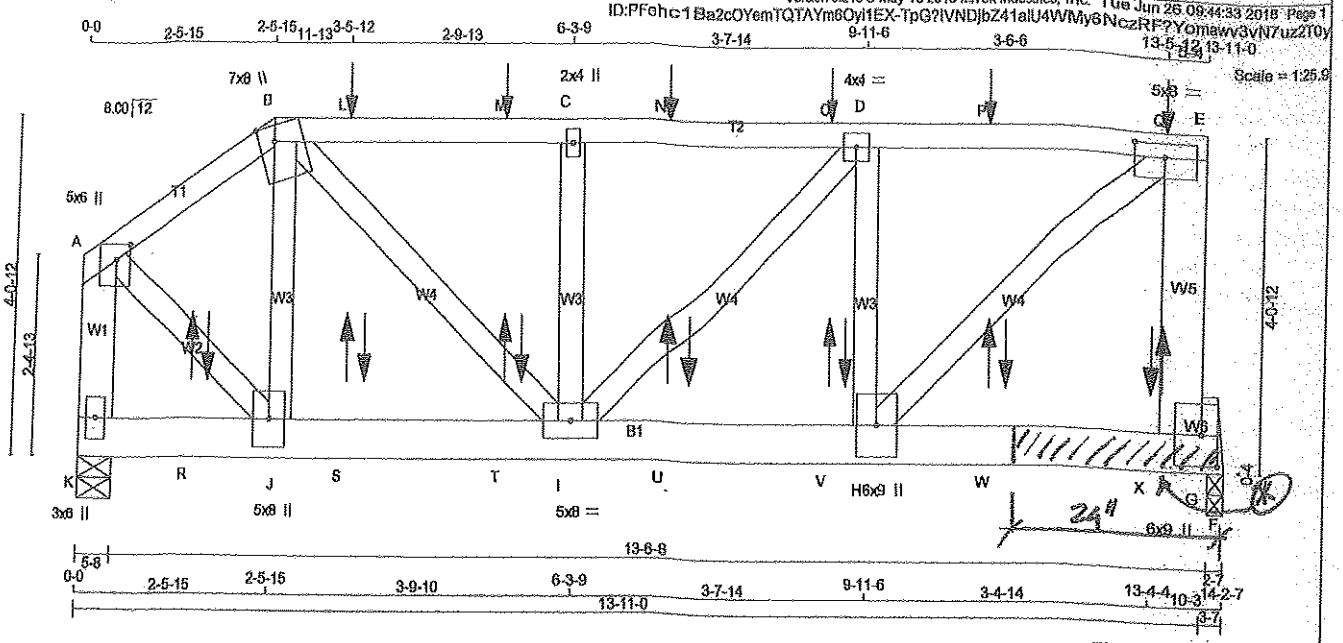

TOTAL WEIGHT = 3 X 83 = 248 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
G - E	2x6	DRY	No.2	SPF
K - A	2x6	DRY	No.2	SPF
K - F	2x6	DRY	2100F 1.8E	SPF
G - F	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- B	1	12	TOP
B- E	1	12	SIDE(91.5)
E- G	2	12	TOP
K- A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
K- F	2	7	SIDE(593.4)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	
2x6	2	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	2.25	2.25
B	TTWW+m	MT20	7.0	8.0	Edge	2.50
C	TMW+w	MT20	2.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TMVW-t	MT20	5.0	8.0	2.00	4.00
G	BMVW+p	MT20	6.0	9.0	4.50	2.50
H	BMWW+t	MT20	6.0	9.0		
I	BMWWW-t	MT20	5.0	8.0		
J	BMWW+t	MT20	5.0	8.0		
K	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DWG NO. TAM T1900651
STRUCTURAL COMPONENT ONLY 1/2

HANGERS NOTES
1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
K	7721	0	7721	0	0	5-8	IN-SX
F	8235	0	8235	0	0	2-7	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	6266	3586 / 0	1349 / 0	0 / 0	0 / 0	1321 / 0	0 / 0
F	6715	3804 / 0	1479 / 0	0 / 0	0 / 0	1432 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.16 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (7)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A- B	-6013 / 0	-84.3	-84.3	0.07 (1)	4.74
B- L	-7933 / 0	-84.3	-84.3	0.14 (1)	4.16
L- M	-7933 / 0	-84.3	-84.3	0.14 (1)	4.16
M- C	-7933 / 0	-84.3	-84.3	0.14 (1)	4.16
C- N	-7933 / 0	-84.3	-84.3	0.12 (1)	4.19
N- O	-7933 / 0	-84.3	-84.3	0.12 (1)	4.19
O- D	-7933 / 0	-84.3	-84.3	0.12 (1)	4.19
D- P	-7121 / 0	-84.3	-84.3	0.13 (1)	4.36
P- Q	-7121 / 0	-84.3	-84.3	0.13 (1)	4.36
Q- E	-7121 / 0	-84.3	-84.3	0.13 (1)	4.36
G- E	-6775 / 0	0.0	0.0	0.34 (1)	8.81
K- A	-7233 / 0	0.0	0.0	0.19 (1)	6.64
K- R	0 / 0	-28.0	-28.0	0.15 (1)	10.00
R- J	0 / 0	-28.0	-28.0	0.15 (1)	10.00
J- S	0 / 4999	-28.0	-28.0	0.23 (1)	10.00
S- T	0 / 4999	-28.0	-28.0	0.23 (1)	10.00
T- I	0 / 4999	-28.0	-28.0	0.23 (1)	10.00
I- U	0 / 7121	-28.0	-28.0	0.36 (1)	10.00
U- V	0 / 7121	-28.0	-28.0	0.36 (1)	10.00
V- H	0 / 7121	-28.0	-28.0	0.36 (1)	10.00
H- W	0 / 0	-28.0	-28.0	0.38 (1)	10.00
W- X	0 / 0	-28.0	-28.0	0.38 (1)	10.00
X- G	0 / 0	-28.0	-28.0	0.38 (1)	10.00
G- F	0 / 0	-28.0	-28.0	0.22 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- B	-14 / 238	0.01 (1)
A- J	0 / 6255	0.34 (1)
H- E	0 / 9725	0.52 (1)
B- I	0 / 4081	0.22 (1)
H- D	-1157 / 0	0.07 (1)
I- C	-297 / 0	0.02 (1)
I- D	0 / 1158	0.06 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
L	3-5-12	-1	-1	73	FRONT	VERT	TOTAL
M	5-5-12	-1	-1	74	FRONT	VERT	TOTAL
N	7-5-12	-1	-1	74	FRONT	VERT	TOTAL
O	9-5-12	-1	-1	74	FRONT	VERT	TOTAL
P	11-5-12	-1	-1	74	FRONT	VERT	TOTAL
Q	13-5-12	-1	-1	59	FRONT	VERT	TOTAL
R	1-5-12	4	1	4	FRONT	VERT	TOTAL
R	1-8-4	-1717	-1717	—	BACK	VERT	TOTAL
S	3-5-12	4	1	4	FRONT	VERT	TOTAL
S	3-8-4	-1717	-1717	—	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
06/26/18
H.J.G. MEYER
PROVINCE OF ONTARIO

DESIGN CRITERIA [M

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.47")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.47")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.34/1.00 (E-G:1) , BC=0.38/1.00 (G-H:1) , WB=0.52/1.00 (E-H:1) , SSI=0.83/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES .

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1097	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (E) (INPUT = 0.90)
JSI METAL= 0.52 (H) (INPUT = 1.00)

PROVIDE 24" LONG SCAB 2X6 SPF #2 ALONG MENBERS (V-W) ,ON ONE SIDE, USING 2 ROWS OF 3 1/2" SPIRAL NAILS @ 4" C/C. (12 NAILS MIN)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285492	T720	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1.1 lbs FACTORED DOWN AND 110.2 lbs FACTORED UP AT 3-5-12, 1.1 lbs FACTORED DOWN AND 110.8 lbs FACTORED UP AT 5-5-12, 1.1 lbs FACTORED DOWN AND 110.8 lbs FACTORED UP AT 7-5-12, 1.1 lbs FACTORED DOWN AND 110.8 lbs FACTORED UP AT 9-5-12, AND 1.1 lbs FACTORED DOWN AND 110.8 lbs FACTORED UP AT 11-5-12, AND 1.1 lbs FACTORED DOWN AND 89.2 lbs FACTORED UP AT 13-5-12 ON TOP CHORD, AND 1.0 lbs FACTORED DOWN AND 5.8 lbs FACTORED UP AT 1-5-12, 2125.6 lbs FACTORED DOWN AT 1-8-4, 1.0 lbs FACTORED DOWN AND 5.8 lbs FACTORED UP AT 3-5-12, 2125.6 lbs FACTORED DOWN AT 3-8-4, 1.0 lbs FACTORED DOWN AND 5.8 lbs FACTORED UP AT 5-5-12, 2125.6 lbs FACTORED DOWN AT 5-8-4, 1.0 lbs FACTORED DOWN AND 5.8 lbs FACTORED UP AT 7-5-12, 2011.3 lbs FACTORED DOWN AT 7-8-4, 1.0 lbs FACTORED DOWN AND 5.8 lbs FACTORED UP AT 9-5-12, 2011.3 lbs FACTORED DOWN AT 9-8-4, 1.0 lbs FACTORED DOWN AND 5.8 lbs FACTORED UP AT 11-5-12, 2011.3 lbs FACTORED DOWN AT 11-8-4, AND 2013.4 lbs FACTORED DOWN AT 13-4-4, AND 1.0 lbs FACTORED DOWN AND 5.8 lbs FACTORED UP AT 13-5-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
T	5-5-12	4	1	4	FRONT	VERT	TOTAL
T	5-8-4	-1717	-1717	—	BACK	VERT	TOTAL
U	7-5-12	4	1	4	FRONT	VERT	TOTAL
U	7-8-4	-1639	-1639	—	BACK	VERT	TOTAL
V	9-5-12	4	1	4	FRONT	VERT	TOTAL
V	9-8-4	-1639	-1639	—	BACK	VERT	TOTAL
W	11-5-12	4	1	4	FRONT	VERT	TOTAL
W	11-8-4	-1639	-1639	—	BACK	VERT	TOTAL
X	13-4-4	-1642	-1642	—	BACK	VERT	TOTAL
X	13-5-12	4	1	4	FRONT	VERT	TOTAL

DWG NO. TAM 7180651
STRUCTURAL
COMPONENT ONLY

2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285876	P6	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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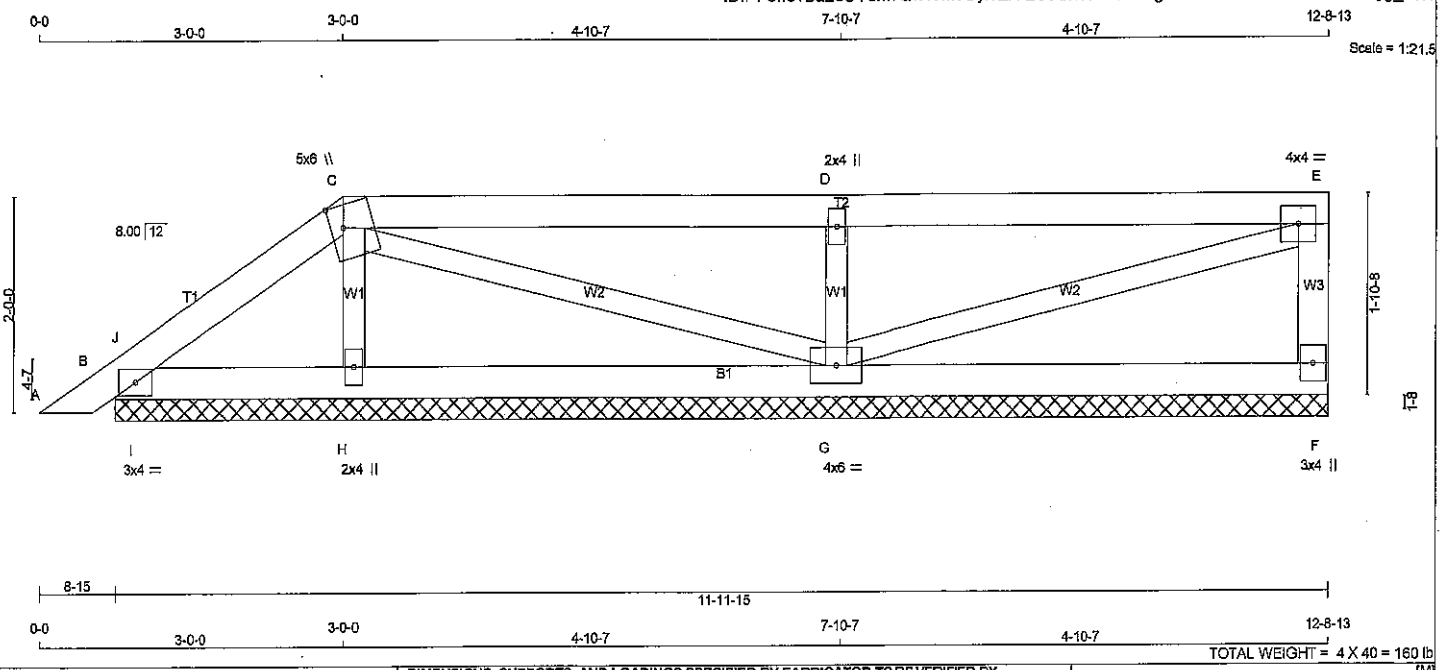


TOTAL WEIGHT = 4 X 40 = 160 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW+m	MT20	5.0	6.0	2.50	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW1-t	MT20	4.0	6.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	210	0	210	0	0	11-11-15	11-11-15
B	194	0	194	0	0	11-11-15	11-11-15
H	321	0	321	0	0	11-11-15	11-11-15
G	677	0	677	0	0	11-11-15	11-11-15

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	173	95 / 0	40 / 0	0 / 0	0 / 0	38 / 0	0 / 0
B	142	108 / 0	14 / 0	0 / 0	0 / 0	22 / 0	0 / 0
H	278	131 / 0	79 / 0	0 / 0	0 / 0	68 / 0	0 / 0
G	550	315 / 0	119 / 0	0 / 0	0 / 0	116 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, H, G

BRACING

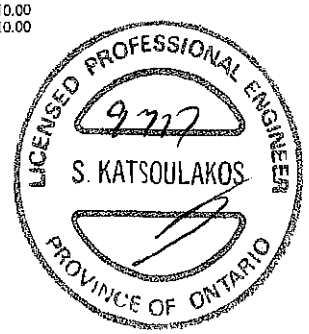
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 14	-84.3	-84.3	0.03 (1)	10.00	H- C	-196 / 0	0.03 (1)
B- J	-32 / 0	-84.3	-84.3	0.01 (1)	6.25	C- G	-27 / 0	0.01 (1)
J- C	-49 / 0	-84.3	-84.3	0.05 (1)	6.25	G- D	-512 / 0	0.07 (1)
C- D	0 / 0	-84.3	-84.3	0.34 (1)	10.00	G- E	0 / 0	0.00 (1)
D- E	0 / 0	-84.3	-84.3	0.34 (1)	10.00	I- J	-121 / 0	0.00 (1)
F- E	-158 / 0	0.0	0.0	0.02 (1)	7.81			
B- I	0 / 38	-28.0	-28.0	0.05 (1)	10.00			
I- H	0 / 38	-28.0	-28.0	0.09 (2)	10.00			
H- G	0 / 25	-28.0	-28.0	0.17 (2)	10.00			
G- F	0 / 0	-28.0	-28.0	0.17 (3)	10.00			



DWG NO. TAM45023-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.34 (C-D:1) , BC=0.17 (G-H:2) , WB=0.07 (D-G:1) , SSI=0.20 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

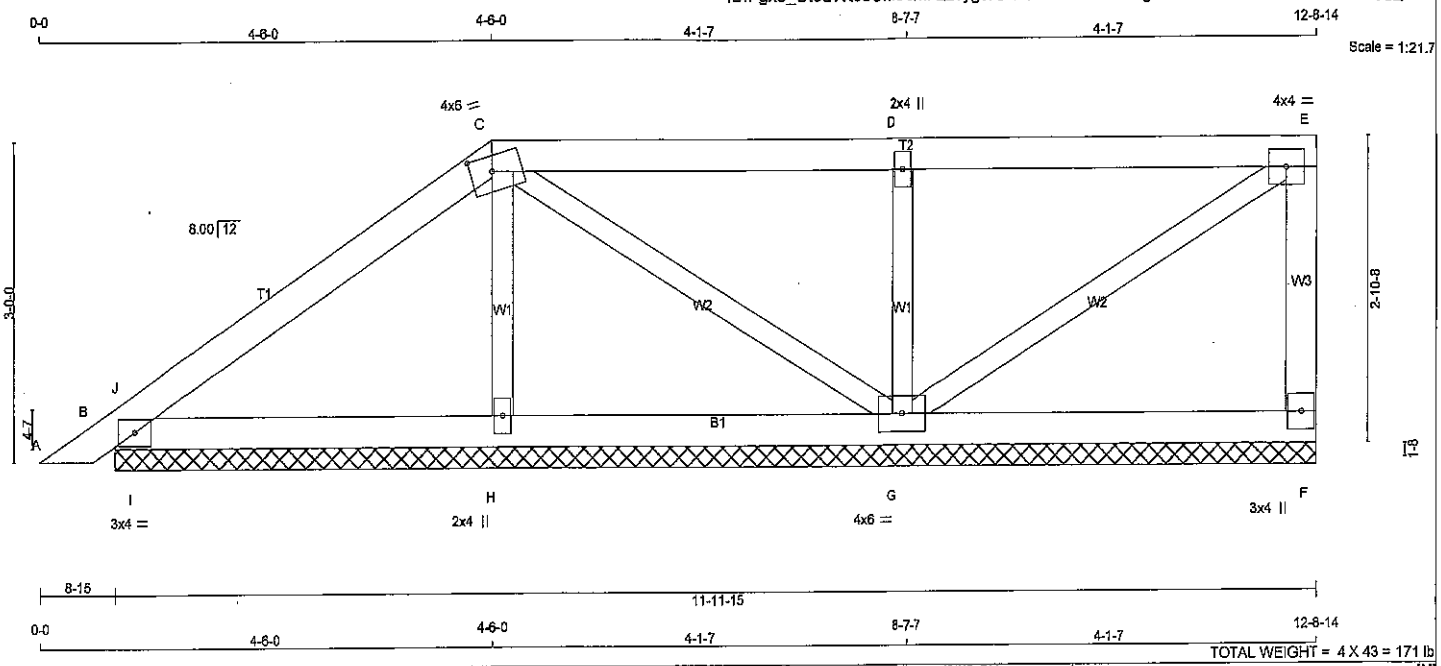
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (D) (INPUT = 0.90)
JSI METAL= 0.09 (D) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285876	P7	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington



TOTAL WEIGHT = 4 X 43 = 171 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW-m	MT20	4.0	6.0	1.75	2.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	4.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW1-t	MT20	4.0	6.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	181	0	181	0	0	11-11-15	11-11-15
B	301	0	301	0	0	11-11-15	11-11-15
H	328	0	328	0	0	11-11-15	11-11-15
G	592	0	592	0	0	11-11-15	11-11-15

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	150	81 / 0	36 / 0	0 / 0	0 / 0	33 / 0	0 / 0
B	232	153 / 0	37 / 0	0 / 0	0 / 0	42 / 0	0 / 0
H	285	133 / 0	82 / 0	0 / 0	0 / 0	70 / 0	0 / 0
G	476	281 / 0	97 / 0	0 / 0	0 / 0	98 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, H, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 14	-84.3	-84.3	0.03 (1)	10.00	H- C	-176 / 0	0.03 (1)
B- J	-73 / 0	-84.3	-84.3	0.04 (1)	6.25	C- G	-75 / 0	0.03 (1)
J- C	-91 / 0	-84.3	-84.3	0.15 (1)	6.25	G- D	-433 / 0	0.07 (1)
C- D	-4 / 0	-84.3	-84.3	0.24 (1)	10.00	G- E	0 / 5	0.00 (3)
D- E	-4 / 0	-84.3	-84.3	0.24 (1)	10.00	I- J	-263 / 74	0.00 (1)
F- E	-133 / 0	0.0	0.0	0.02 (1)	7.81			
B- I	0 / 71	-28.0	-28.0	0.13 (1)	10.00			
I- H	0 / 71	-28.0	-28.0	0.13 (1)	10.00			
H- G	0 / 64	-28.0	-28.0	0.12 (2)	10.00			
G- F	0 / 0	-28.0	-28.0	0.12 (3)	10.00			

DWG NO. YAM 45028-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.24 (C-D:1) , BC=0.13 (H-I:1) , WB=0.07 (D-G:1) , SSI=0.22 (B-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1666

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (D) (INPUT = 0.90)
JSI METAL= 0.08 (D) (INPUT = 1.00)

LUL/LUS/LJS/HUS/HHUS/HGUS

Standard and Double-Shear Joist Hangers

This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 258–259.

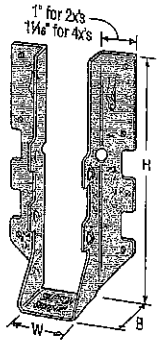
Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 20–24.

Installation:

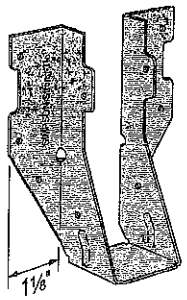
- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

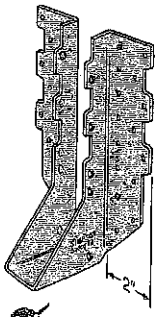
- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on p. 126.



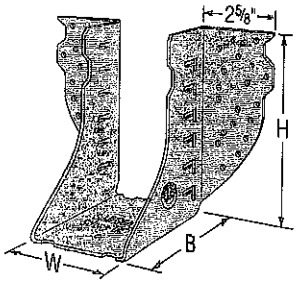
LUS28



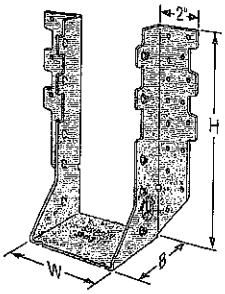
LU26L



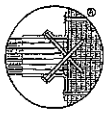
HUS210
(HUS26, HUS28, and HHUS similar)



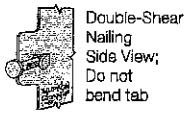
HGUS28-2



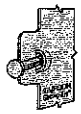
HHUS210-2



Double-Shear Nailing Top View



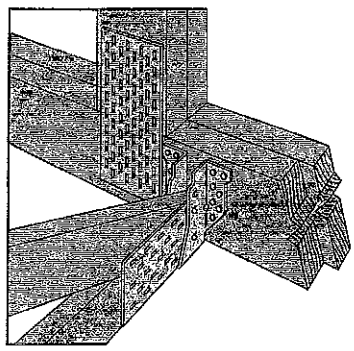
Double-Shear Nailing Side View; Do not bend tab

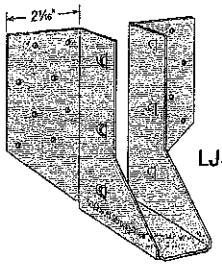


Dome Double-Shear Nailing Side View *(available on some models)*

U.S. Patent 5,603,580

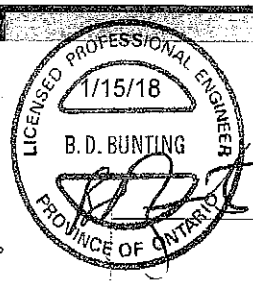
Typical HUS26 Installation with Reduced Heel Height
(Truss Designer to provide fastener quantity for connecting multiple members together)





LJS26DS

LUL/LUS/LJS/HUS/HHUS/HGUS



HHUS/HGUS

See Hanger Options information on pp. 125–127.

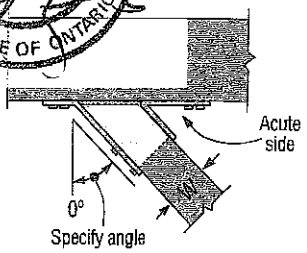
HHUS — Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.62 of the table value
- The joist must be bevel-cut to allow for double-shear nailing

HGUS — Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Joist	Down Resistance	Uplift
W < 2"	Bevel or square cut	0.62 of table value	0.46 of table value
2" < W < 6"	Bevel cut	0.67 of table value	0.41 of table value
2" < W < 6"	Square cut	0.46 of table value	0.41 of table value
W > 6"	Bevel cut	0.75 of table value	0.41 of table value



Top View HHUS Hanger Skewed Right
(joist must be bevel cut)
All joist nails installed on the outside angle *(non-acute side)*.

Standard and Double-Shear Joist Hangers (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.) W	H	B	d_e[3]	Fasteners Header	Joist	Factored Resistance D.Fir-L Uplift (K_D = 1.15) lb. / kN	D.Fir-L Normal (K_D = 1.00) lb. / kN	S-P-F Uplift (K_D = 1.15) lb. / kN	S-P-F Normal (K_D = 1.00) lb. / kN
Single 2x Sizes											
LUS24	18	1⁹⁄₁₆	3⅛	1¾	2¼	(4) 10d	(2) 10d	710 / 3.16	1625 / 7.23	645 / 2.87	1155 / 5.14
LU24L	22	1⁹⁄₁₆	3	1⅝	2¹¹⁄₁₆	(4) 10d	(2) 10d x 1½"	360 / 1.60	1020 / 4.54	320 / 1.42	725 / 3.22
LU26L	22	1⁹⁄₁₆	5	1⅝	4⅝	(6) 10d	(4) 10d x 1½"	720 / 3.20	1605 / 7.14	645 / 2.87	1140 / 5.07
LUS26	18	1⁹⁄₁₆	4¾	1¾	3¾	(4) 10d	(4) 10d	1420 / 6.32	2170 / 9.65	1290 / 5.74	1630 / 7.25
HUS26	16	1⅝	5⅜	3	3¹⁵⁄₁₆	(14) 16d	(6) 16d	2705 / 11.30	4940 / 21.97	2065 / 9.20	3875 / 17.24
LJS26DS	18	1⁹⁄₁₆	5	3½	4⅝	(16) 16d	(6) 16d	2055 / 9.14	4265 / 18.97	1460 / 6.49	4115 / 18.31
HGUS26	12	1⅝	5⅜	5	4⅛	(20) 16d	(8) 16d	2685 / 11.96	6625 / 29.51	2685 / 11.96	5700 / 25.35
LU26L	20	1⁹⁄₁₆	6¾	1⅝	5⅞	(8) 10d	(6) 10d x 1½"	1140 / 5.07	2185 / 9.72	1020 / 4.54	1550 / 6.89
LUS28	18	1⁹⁄₁₆	6⅝	1¾	3¾	(6) 10d	(4) 10d	1420 / 6.32	2520 / 11.21	1290 / 5.74	1790 / 7.96
HUS28	16	1⅝	7¹⁄₁₆	3	6⅛	(22) 16d	(8) 16d	3605 / 16.04	5365 / 23.86	2675 / 11.90	4345 / 19.33
HGUS28	12	1⅝	7⅛	5	6⅛	(36) 16d	(12) 16d	3310 / 14.74	7675 / 34.19	3310 / 14.74	6900 / 30.73
LU210L	20	1⁹⁄₁₆	8	1⅝	7⅝	(10) 10d	(6) 10d x 1½"	1140 / 5.07	2495 / 11.10	1020 / 4.54	1770 / 7.87
LUS210	18	1⁹⁄₁₆	7¹³⁄₁₆	1¾	3⅞	(8) 10d	(4) 10d	1420 / 6.32	2785 / 12.39	1290 / 5.74	2210 / 9.83

1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.
3. d_e is the distance from the bearing seat to the top joist nail.
4. Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-C-N10TRSSCN and/or see installation notes.
5. **Nails:** 16d = 0.162" dia. x 3½" long. See pp. 27–28 for other nail sizes and information.

Face-Mount Hangers

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance			
								D.Fir-L		S-P-F	
		W	H	B	d_e[3]	Header	Joist	Uplift (K_D = 1.15)	Normal (K_D = 1.00)	Uplift (K_D = 1.15)	Normal (K_D = 1.00)
								lb.	lb.	lb.	lb.
								kN	kN	kN	kN
Double 2x Sizes											
LUS24-2	18	3⅛	3⅛	2	1½	(4) 16d	(2) 16d	835	2020	590	1435
								3.71	8.99	2.62	6.38
LUS26-2	18	3⅛	4⅞	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
								7.65	11.54	6.87	8.54
HHUS26-2	14	3⁵⁄₁₆	5⅜	3	3¹⁵⁄₁₆	(14) 16d	(6) 16d	2850	7335	2065	5205
								12.68	32.63	9.20	23.15
HGUS26-2	12	3⁵⁄₁₆	5⁷⁄₁₆	4	4⅛	(20) 16d	(8) 16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
LUS28-2	18	3⅛	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
								7.65	14.79	6.87	11.45
HHUS28-2	14	3⁵⁄₁₆	7⁵⁄₁₆	3	6⅛	(22) 16d	(8) 16d	3765	8940	2675	6345
								16.75	39.77	11.90	28.22
HGUS28-2	12	3⁵⁄₁₆	7³⁄₁₆	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
LUS210-2	18	3⅛	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
								11.48	20.02	10.32	14.21
HHUS210-2	14	3⁵⁄₁₆	9³⁄₁₆	3	8	(30) 16d	(10) 16d	4670	9660	4235	7000
								20.77	42.97	18.84	31.14
HGUS210-2	12	3⁵⁄₁₆	9³⁄₁₆	4	8⅛	(46) 16d	(16) 16d	6840	14015	4855	10270
								30.43	62.34	21.60	45.69
Triple 2x Sizes											
HGUS26-3	12	4¹⁵⁄₁₆	5½	4	4⅛	(20) 16d	(8) 16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
HGUS28-3	12	4¹⁵⁄₁₆	7¼	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
HHUS210-3	14	4¹⁵⁄₁₆	9	3	7¹⁵⁄₁₆	(30) 16d	(10) 16d	4670	9670	4235	6865
								20.77	43.02	18.84	30.54
HGUS210-3	12	4¹⁵⁄₁₆	9¼	4	8⅜	(46) 16d	(16) 16d	6840	14645	4855	10400
								30.43	65.14	21.60	46.26
Quadruple 2x Sizes											
HGUS26-4	12	6⁵⁄₁₆	5⁷⁄₁₆	4	4⅛	(20) 16d	(8) 16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
HGUS28-4	12	6⁵⁄₁₆	7³⁄₁₆	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
HHUS210-4	14	6⅛	8⅞	3	7¹³⁄₁₆	(30) 16d	(10) 16d	4670	10155	4235	7210
								20.77	45.17	18.84	32.07
HGUS210-4	12	6⁵⁄₁₆	9³⁄₁₆	4	8⅛	(46) 16d	(16) 16d	6840	14645	4855	10400
								30.43	65.14	21.60	46.26
HGUS212-4	12	6⁵⁄₁₆	10⅝	4	10⅛	(56) 16d	(20) 16d	7640	14995	5425	10645
								33.98	66.70	24.13	47.35
HGUS214-4	12	6⁵⁄₁₆	12⅝	4	11⅛	(66) 16d	(22) 16d	10130	16400	7195	11645
								45.06	72.95	32.00	51.80
4x Sizes											
LUS46	18	3⁵⁄₁₆	4¾	2	3⅞	(4) 16d	(4) 16d	1720	2595	1545	1920
								7.65	11.54	6.87	8.54
HHUS46	14	3⅝	5¼	3	3¹⁵⁄₁₆	(14) 16d	(6) 16d	2540	7335	2065	5205
								11.30	32.63	9.20	23.15
HGUS46	12	3⅝	5¼	4	4⅛	(20) 16d	(8) 16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
LUS48	18	3⁵⁄₁₆	6¾	2	3⅞	(6) 16d	(4) 16d	1720	3325	1545	2575
								7.65	14.79	6.87	11.45
HHUS48	14	3⅝	7⅛	3	6⅛	(22) 16d	(8) 16d	3765	8940	2675	6345
								16.75	39.77	11.90	28.22
HGUS48	12	3⅝	7¼	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
LUS410	18	3⁵⁄₁₆	8¾	2	5⁵⁄₁₆	(8) 16d	(6) 16d	2580	4500	2320	3195
								11.48	20.02	10.32	14.21
HGUS410	12	3⅝	9	4	8⅛	(46) 16d	(16) 16d	6840	14015	4855	10270
								30.43	62.34	21.60	45.69
HGUS412	12	3⅝	10⅞	4	10⅛	(56) 16d	(20) 16d	7640	14995	5425	10645
								33.98	66.70	24.13	47.35
HGUS414	12	3⅝	12⅞	4	11⅛	(66) 16d	(22) 16d	10130	16400	7195	11645
								45.06	72.95	32.00	51.80

See footnotes on p. 258.

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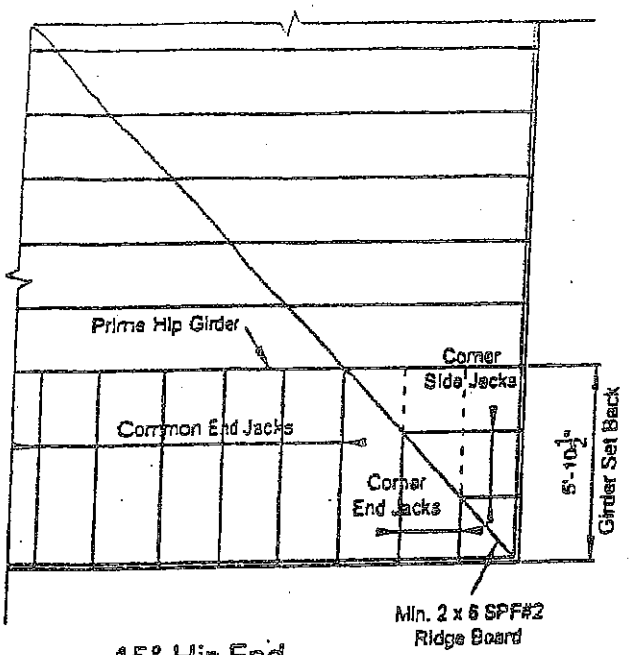

45° Hip End

LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2

BOTTOM CHORD : 2 x 4 SPF#2

WEBS : 2 x 3 SPF#2

UNLESS OTHERWISE SHOWN

DESIGN LOAD:

TOP CHORD LIVE LOAD	:	34.8 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD	:	44.8 P.S.F.

DWG NO TAM 3495.14 R1
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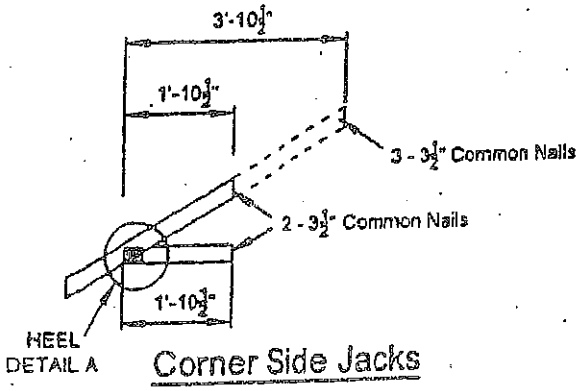
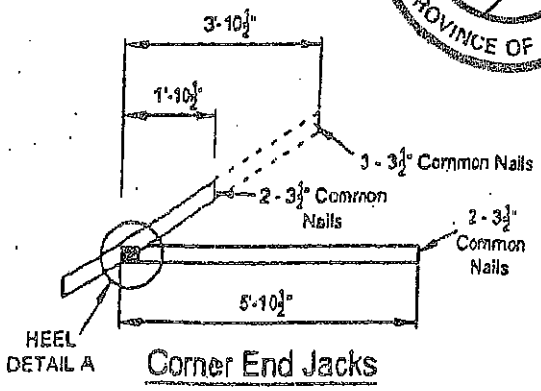
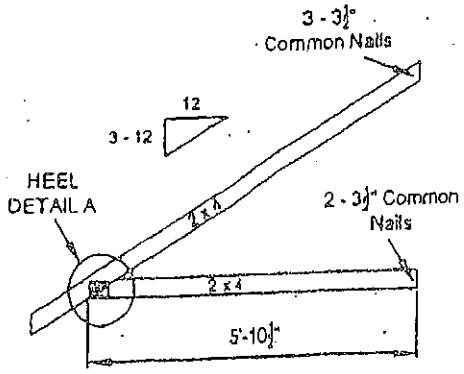
1-2024
S. KATSOULAKOS
PROVINCE OF ONTARIO



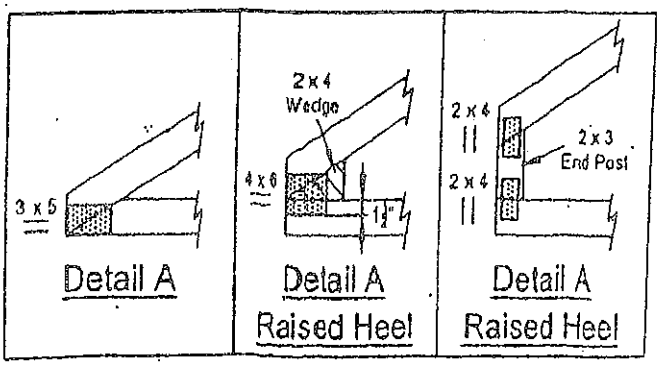
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

MICRO CITY
ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

DESIGN LOAD:

TOP CHORD LIVE LOAD	: 40.5 P.S.F.
TOP CHORD DEAD LOAD	: 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	: 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	: 7.0 P.S.F.
TOTAL LOAD	: 50.5 P.S.F.

DWG NO TAM 3504.1
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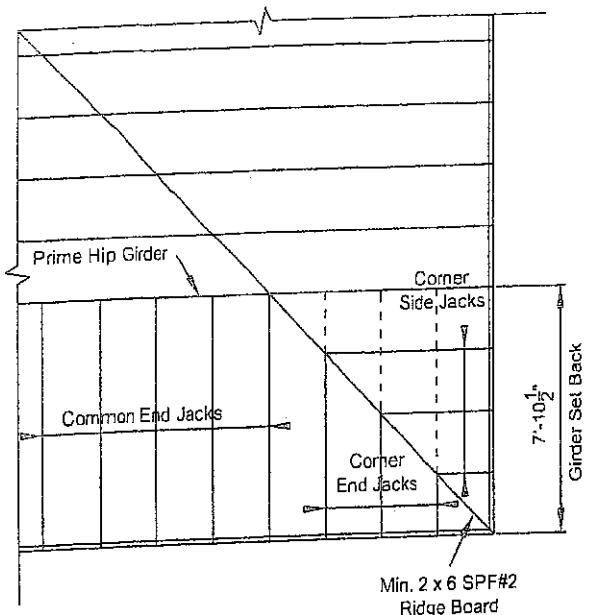
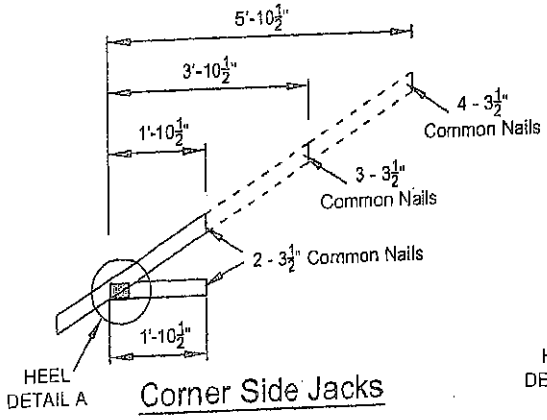
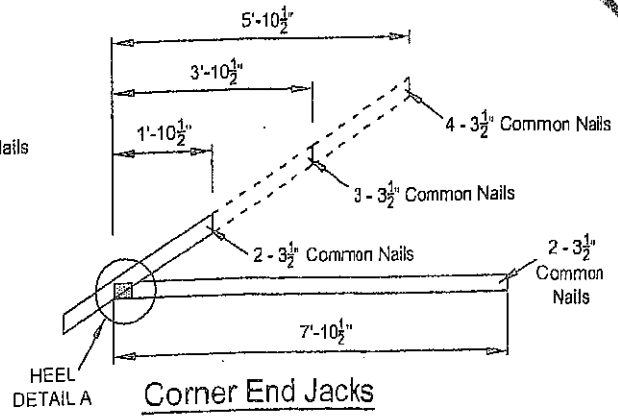


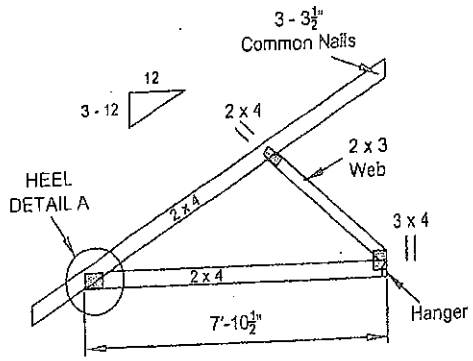
45° Hip End



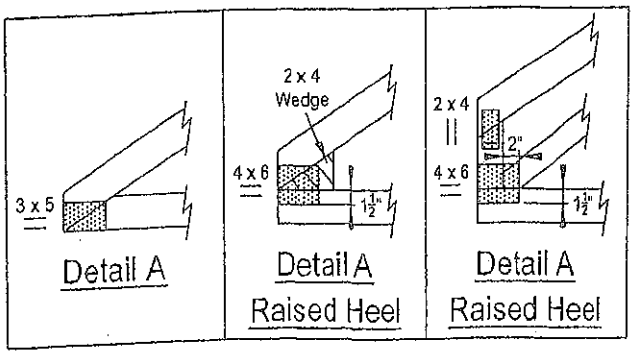
Corner Side Jacks



Corner End Jacks



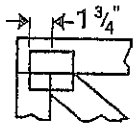
Common End Jacks



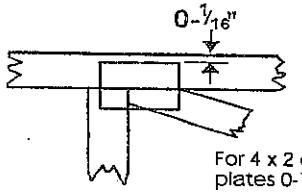
NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

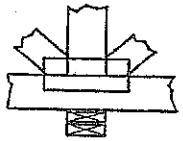
4 X 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

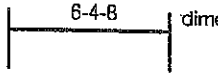
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses

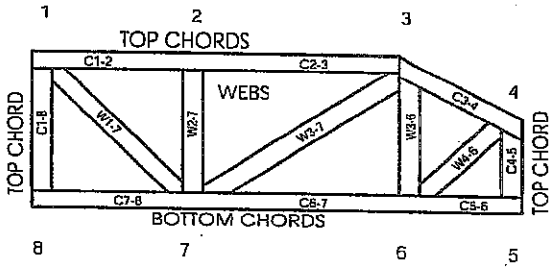
DSB-89: Design Standard for Bracing.

BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

MiTek

POWER TO PERFORM.™

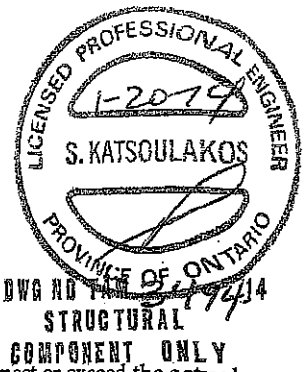
MiTek Engineering Reference Sheet: MII-7473C rev. 10-'08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

Micro City Engineering Services Inc.
(BCIN: 26064; FIRM BCIN: 29991)
RR #1, Po Box 61
Glencoe, Ontario
N0L 1M0
(519) 287 - 2242; Fax: (519) 287 - 5750 (Call)



Responsibilities:

Micro City Engineering Services is responsible for the design of trusses as individual components.

It is the responsibilities of others to ascertain that the design loads utilized on this (these) drawing(s) meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdiction over such decisions.

All dimensions are to be verified by the owner, contractor, architect, or other authority having input over such decisions prior to truss component manufacture. At no time shall Micro City Engineering Services Inc. or its employees be responsible for dimension errors.

Micro City Engineering Services Inc. bears no responsibility for the erection of any truss components. Persons erecting truss components are cautioned to seek professional advice regarding temporary and permanent bracing systems and to be **totally** familiar with all aspects of truss erection prior to proceeding on **any** truss component erection job. Any bracing shown on Micro City Engineering Services Inc. or Tamarack Roof Trusses Inc. sealed or unsealed truss component drawings is specified for the single truss component in question and is identified as an integral part of the design for that particular truss component but is **not** meant to represent the only required bracing for that particular truss component when installed as a component in a series of truss components in a roof truss system.

It is the truss manufacturer's responsibility to ensure that trusses are manufactured in accordance with Micro City Engineering Services Inc. specifications outlined below:

SPECIFICATIONS:

Truss components sealed by Micro City Engineering Services Inc. must conform to the relevant sections of the current Building Code of Ontario and Canada (Part 4 or Part 9) or the current Farm Building Code of Canada in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the Truss Plate Institute of Canada (TPIC). All unit lumber and nailing stresses identified on truss component design drawings and/or used in the design of individual truss components shall conform to the current CSA Wood Design standard identified in the current Building Code and TPIC Design Standards.

The lumber used to manufacture any truss component is to conform to the specified size and grade identified on the truss drawing.

The lumber used in the manufacture of any truss component is not to exceed 19% during its service use unless specifically noted on the truss drawing.

The lumber used in the manufacture of any truss component is not to be treated with any chemicals during its service life unless specifically noted on the truss drawing.

Connector plates shall be applied to both faces of the truss component at each joint and shall be positioned exactly as specified.

The top chord of any truss component is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the sealed truss component drawing but not exceeding 24" o/c (Part 9 design) and not exceeding 48" o/c (Part 4 or Agricultural design).

When a truss component is to be installed with no rigid ceiling attached directly to the bottom chord, then the bottom chord is to be laterally braced at intervals not exceeding 3m (or 10'-0").

All sealed or unsealed truss component drawings provided by Micro City Engineering Services Inc. Or Tamarack Roof Trusses Inc. should be read in conjunction with the following:

Warning-Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473C rev 10-'08 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer - not the truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component Safety Information available from the Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA, 22314.