

ASPHALT SHINGLES
12" FINISH O.H.
R.T.M.C
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:

HGUS26-2 -(XX)
LJS26DS -(V)

8/12 PITCHES (TYP.)
UNLESS NOTED

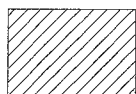
DENOTES:

* -1'-6"-0 HIGHER PLATE

ALL B- 2-2X10 BEAMS (FLUSH)

T-170534

DENOTES:
CONVENTIONAL
FRAMING



HIGHGROVE-5E
EL:1 - UNIT1

285746

HIGHGROVE-4
EL:1- UNIT2

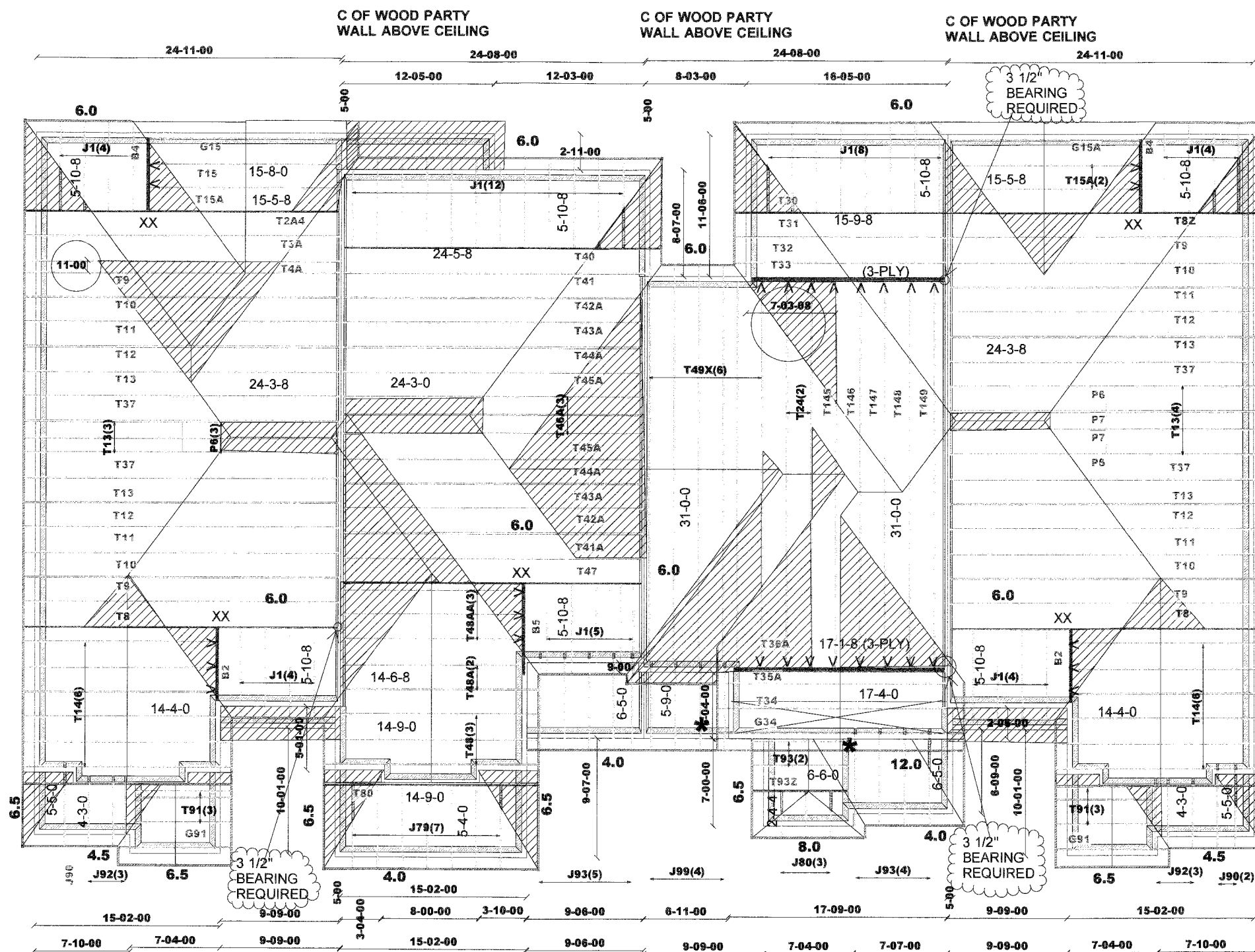
285350

HIGHGROVE-3
EL:1A (REV)- UNIT3

285748

HIGHGROVE-5E
EL:1(REV) - UNIT4

285749



ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART 9 OF THE OBC LATEST EDITION
ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4" SPF@24" o.c.
WITH A 2"X4" SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.
POSTS LONGER THAN 6' TO BE Laterally Braced so that the distance
between end points and between rows of bracing does not exceed 6'.

DESIGN LOADS:

SNOW LOAD 25.6 PSF
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF



Job Track: 43954
Layout ID: 285745
Plan Log: 93446

Builder / Location:

GREENPARK HOMES / WATERDOWN

Model / Elevation:

BLOCK209 / UNIT1TO4 (REVISED)

Mitek ver 7.5.0

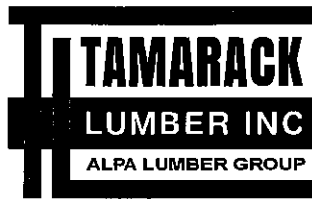
Project:

RUSSEL GARDENS PHASE 2

Date: 6/19/2018

Designer: sonny

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Delivery Shiplist

DATE	04/18/18
SALES REP	Mario

JOB TRACK: 43954	LAYOUT ID: 285746	LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH 2	SUB-BUILDER:	
MODEL: HIGHGROVE 5E	ELEVATION: 1-UNIT1	

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY PLY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
						TOP	BOT					
	1	T2A4 HIP GIRDER	8.00 0.00	24-03-08	04-01-04	2 X 6	2 X 6	01-03-08 00-00-00	01-04-13 01-06-08	142.19 87.67		
	1	T3A HIP	8.00 0.00	24-03-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	94.81 60.17		
	1	T4A HIP	8.00 0.00	24-03-08	06-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	103.53 66.33		
	1	T8 HALF HIP	8.00 0.00	24-03-08	04-01-04	2 X 6	2 X 6	01-03-08 00-00-00	01-04-13 04-01-04	143.26 88.34		
	2	T9 HALF HIP	8.00 0.00	24-03-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-01-04	197.10 123.66		
	2	T10 HALF HIP	8.00 0.00	24-03-08	06-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 06-01-04	212.66 133.34		
	2	T11 HALF HIP	8.00 0.00	24-03-08	07-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 07-01-04	225.56 142.68		
	2	T12 HALF HIP	8.00 0.00	24-03-08	08-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 08-01-04	234.64 146.34		
	5	T13 HALF HIP	8.00 0.00	24-03-08	09-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 09-01-04	614.40 379.15		
	2	T37 HALF HIP	8.00 0.00	24-03-08	10-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 10-01-04	257.54 159.34		
	1	T15 COMMON	8.00 0.00	15-08-00	06-07-08	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	66.76 42.00		
	1	G15 COMMON	8.00 0.00	15-08-00	06-07-08	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	69.72 45.00		
	1	T15A COMMON	8.00 0.00	15-05-08	06-07-08	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	64.50 41.33		
	6	T14 COMMON	8.00 0.00	14-04-00	06-02-02	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	370.56 234.00		
	3	T91 COMMON	6.50 0.00	06-06-00	02-04-09	2 X 4	2 X 4	01-03-08 01-03-08	00-07-07 00-07-07	66.60 45.00		
	1	G91 COMMON	6.50 0.00	06-06-00	02-04-09	2 X 4	2 X 4	01-03-08 01-03-08	00-07-07 00-07-07	23.39 16.00		
	3	P6 PIGGYBACK	8.00 0.00	11-11-15	01-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 01-10-08	120.33 77.01		
	8	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	134.32 85.36		



Delivery Shiplist

DATE	04/18/18
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 285746 LOCATION:
 BUILDER: GREENPARK - RUSSELL GARDENS PH 2 SUB-BUILDER:
 MODEL: HIGHGROVE 5E ELEVATION: 1-UNIT1

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	J90 MONOPITCH	4.50 0.00	05-05-00	02-05-00	2 X 4	2 X 4	01-03-08 00-00-00	00-04-10 02-05-00	14.47 9.33		
	3	J92 JACK	4.50 0.00	04-03-00	01-11-12	2 X 4	2 X 4	01-03-08 00-00-00	00-04-10 01-11-12	35.16 24.00		

TOTAL # TRUSS= 47.00

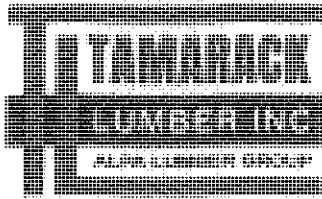
TOTAL BFT OF ALL TRUSSES=

2006.05 BFT. TOTAL WEIGHT OF ALL TRUSSES= 3191.50 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
5	Hangers	LJS26DS	

TOTAL # ITEMS= 7.00



Delivery Shiplist

DATE	06/22/18
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 285350 LOCATION:
 BUILDER: GREENPARK - RUSSELL GARDENS PH SUB-BUILDER:
 MODEL: HIGHGROVE 4 ELEVATION: EL:1- UNIT2 OR UNIT5

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY PLY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER TOP BOT	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	1	T40 HALF HIP	8.00 0.00	24-05-08	04-01-04	2 X 6 2 X 6	01-03-08 00-00-00	01-04-13 04-01-04	136.90 84.33		
	1	T41 HALF HIP	8.00 0.00	24-05-08	05-01-04	2 X 4 2 X 4	01-03-08 00-00-00	01-04-13 05-01-04	99.05 61.83		
	1	T41A HALF HIP	8.00 0.00	24-03-00	05-01-04	2 X 4 2 X 4	00-00-00 00-00-00	01-06-08 05-01-04	96.77 61.17		
	2	T42A HALF HIP	8.00 0.00	24-03-00	06-01-04	2 X 4 2 X 4	00-00-00 00-00-00	01-06-08 06-01-04	209.24 132.00		
	2	T43A HALF HIP	8.00 0.00	24-03-00	07-01-04	2 X 4 2 X 4	00-00-00 00-00-00	01-06-08 07-01-04	222.10 141.34		
	2	T44A HALF HIP	8.00 0.00	24-03-00	08-01-04	2 X 4 2 X 4	00-00-00 00-00-00	01-06-08 08-01-04	231.32 146.34		
	2	T45A HALF HIP	8.00 0.00	24-03-00	09-01-04	2 X 4 2 X 4	00-00-00 00-00-00	01-06-08 09-01-04	242.42 150.34		
	3	T46A HALF HIP	8.00 0.00	24-03-00	10-01-04	2 X 4 2 X 4	00-00-00 00-00-00	01-06-08 10-01-04	381.24 237.00		
	1	T47 FLAT GIRDER	0.00 0.00	24-03-00	04-01-04	2 X 6 2 X 6	00-00-00 00-00-00	04-01-04 04-01-04	145.78 90.67		
	3	T48 COMMON	8.00 0.00	14-09-00	06-03-13	2 X 4 2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	189.99 117.00		
	2	T48A COMMON	8.00 0.00	14-06-08	06-03-13	2 X 4 2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	122.14 76.66		
	3	T48AA COMMON	8.00 0.00	14-08-08	06-03-13	2 X 4 2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	183.21 114.99		
	1	T80 HIP GIRDER	6.50 0.00	14-09-00	02-01-04	2 X 4 2 X 4	01-03-08 01-03-08	00-07-07 00-07-07	52.40 33.33		
	17	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4 2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	285.43 181.39		
	7	J79 JACK-OPEN	4.00 0.00	05-04-00	02-01-04	2 X 4 2 X 4	01-03-08 00-00-00	00-03-15 02-01-04	99.05 65.31		
	5	J93 MONOPITCH	4.00 0.00	06-05-00	02-05-10	2 X 4 2 X 4	01-03-08 00-00-00	00-03-15 02-05-10	90.80 58.35		

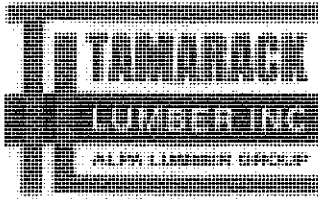
TOTAL # TRUSS= 53

TOTAL BFT OF ALL TRUSSES=

1752.05 BFT. TOTAL WEIGHT OF ALL TRUSSES= 2787.84 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
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Delivery Shiplist

DATE	06/22/18
SALES REP	Mario

JOB TRACK: 43954

LAYOUT ID: 285350

LOCATION:

BUILDER: GREENPARK - RUSSELL GARDENS PH

SUB-BUILDER:

MODEL: HIGHGROVE 4

ELEVATION: EL:1- UNIT2 OR UNIT5

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	HGUS26-2	
3	Hangers	LJS26DS	

TOTAL # ITEMS= 4



Delivery Shiplist

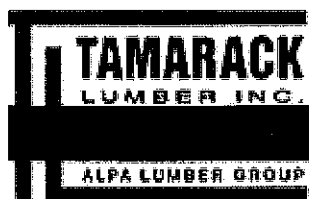
DATE	09/05/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 285748 LOCATION:
 BUILDER: GREENPARK - RUSSELL GARDENS PH2 SUB-BUILDER:
 MODEL: HIGHGROVE 3 ELEVATION: 1A (REV)- UNIT3

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	6	T49X COMMON	6.00 0.00	31-00-00	08-08-12	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 00-09-08	717.12 444.00		
	2	T24 COMMON	6.00 0.00	31-00-00	08-11-00	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	243.10 153.00		
	1	T145 FLAT	0.00 0.00	31-00-00	05-11-13	2 X 4	2 X 4	00-00-00 00-00-00	05-11-13 05-11-13	133.24 85.17		
	1	T146 HALF HIP	6.00 0.00	31-00-00	07-03-13	2 X 4	2 X 4	00-00-00 00-00-00	06-11-00 07-03-13	140.32 86.67		
	1	T147 HIP	6.00 0.00	31-00-00	08-07-13	2 X 4	2 X 4	00-00-00 00-00-00	06-11-00 02-07-00	153.53 98.83		
	1	T148 HIP	6.00 0.00	31-00-00	09-11-13	2 X 4	2 X 4	00-00-00 00-00-00	06-11-00 02-07-00	155.90 98.17		
	1	T149 HIP	6.00 0.00	31-00-00	11-03-13	2 X 4	2 X 4	00-00-00 00-00-00	06-11-00 02-07-00	162.00 100.00		
	1	T30 HALF HIP	8.00 0.00	15-09-08	04-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-04-13 04-01-04	77.31 47.50		
	1	T31 HALF HIP	8.00 0.00	15-09-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-01-04	67.94 43.00		
	1	T32 HALF HIP	8.00 0.00	15-09-08	06-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 06-01-04	74.63 46.67		
	1 3 Ply	T33 HALF HIP	8.00 0.00	15-09-08	06-08-12	2 X 6	2 X 6	00-00-00 00-00-00	01-04-13 06-08-12	341.19 201.00		
	1	T34 COMMON	8.00 0.00	17-04-00	07-02-02	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	73.02 46.17		
	1	G34 COMMON	8.00 0.00	17-04-00	07-02-02	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	76.31 49.17		
	1	T35A COMMON	8.00 0.00	17-01-08	08-08-02	2 X 4	2 X 4	01-03-08 00-00-00	02-10-13 03-00-08	78.95 50.50		
	1 3 Ply	T36A COMMON	8.00 0.00	17-01-08	08-08-02	2 X 6	2 X 6	00-00-00 00-00-00	02-10-13 03-00-08	375.66 237.99		
	2	T93 COMMON	6.50 0.00	06-06-00	02-04-09	2 X 4	2 X 4	01-03-08 01-03-08	00-07-07 00-07-07	44.40 30.00		
	1	T93Z COMMON	6.50 0.00	06-06-00	02-04-09	2 X 4	2 X 4	01-03-08 01-03-08	00-07-07 00-07-07	22.20 15.00		
	8	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	134.32 85.36		



Delivery Shiplist

DATE	09/05/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 285748 LOCATION:
 BUILDER: GREENPARK - RUSSELL GARDENS PH¹ SUB-BUILDER:
 MODEL: HIGHGROVE 3 ELEVATION: 1A (REV)- UNIT3

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	3	J80 JACK-OPEN	8.00 0.00	02-04-04	02-04-07	2 X 4	2 X 4	01-03-08 00-00-00	00-09-10 02-04-07	24.93 18.00		
	4	J93 MONOPITCH	4.00 0.00	06-05-00	02-05-10	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-05-10	72.64 46.68		
	4	J99 MONOPITCH	4.00 0.00	05-09-00	02-02-15	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-02-15	60.52 37.32		

TOTAL # TRUSS= 47

TOTAL BFT OF ALL TRUSSES=

2020.20 BFT. TOTAL WEIGHT OF ALL TRUSSES= 3229.23 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
16	Hangers	LJS26DS	

TOTAL # ITEMS= 16

DATE	04/18/18
SALES REP	Mario



Delivery Shiplist

JOB TRACK: 43954	LAYOUT ID: 285749	LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH 2	SUB-BUILDER:	
MODEL: HIGHGROVE 5E	ELEVATION: 1(REV)- UNIT: 4	

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY PLY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
						TOP	BOT					
	1	T8 HALF HIP	8.00 0.00	24-03-08	04-01-04	2 X 6	2 X 6	01-03-08 00-00-00	01-04-13 04-01-04	143.26 88.34		
	1	T8Z HALF HIP	8.00 0.00	24-03-08	04-01-04	2 X 6	2 X 6	01-03-08 00-00-00	01-04-13 04-01-04	143.26 88.34		
	2	T9 HALF HIP	8.00 0.00	24-03-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-01-04	197.10 123.66		
	2	T10 HALF HIP	8.00 0.00	24-03-08	06-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 06-01-04	212.66 133.34		
	2	T11 HALF HIP	8.00 0.00	24-03-08	07-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 07-01-04	225.56 142.68		
	2	T12 HALF HIP	8.00 0.00	24-03-08	08-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 08-01-04	234.64 146.34		
	6	T13 HALF HIP	8.00 0.00	24-03-08	09-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 09-01-04	737.28 454.98		
	2	T37 HALF HIP	8.00 0.00	24-03-08	10-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 10-01-04	243.36 152.00		
	2	T15A COMMON	8.00 0.00	15-05-08	06-07-08	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	129.00 82.66		
	1	G15A COMMON	8.00 0.00	15-05-08	06-07-08	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	67.52 44.33		
	6	T14 COMMON	8.00 0.00	14-04-00	06-02-02	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	370.56 234.00		
	3	T91 COMMON	6.50 0.00	06-06-00	02-04-09	2 X 4	2 X 4	01-03-08 01-03-08	00-07-07 00-07-07	66.60 45.00		
	1	G91 COMMON	6.50 0.00	06-06-00	02-04-09	2 X 4	2 X 4	01-03-08 01-03-08	00-07-07 00-07-07	23.39 16.00		
	2	P6 PIGGYBACK	8.00 0.00	11-11-15	01-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-00-00 02-00-00	80.22 51.34		
	2	P7 PIGGYBACK	8.00 0.00	11-11-15	02-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-00-00 03-00-00	85.52 56.00		
	8	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	134.32 85.36		
	1	J90 MONOPITCH	4.50 0.00	05-05-00	02-05-00	2 X 4	2 X 4	01-03-08 00-00-00	00-04-10 02-05-00	14.47 9.33		
	3	J92 MONOPITCH	4.50 0.00	04-03-00	01-11-12	2 X 4	2 X 4	01-03-08 00-00-00	00-04-10 01-11-12	35.16 24.00		

TOTAL # TRUSS= 47.00

TOTAL BFT OF ALL TRUSSES=

1977.70 BFT.

TOTAL WEIGHT OF ALL TRUSSES=

3143.88 LBS.



Delivery Shiplist

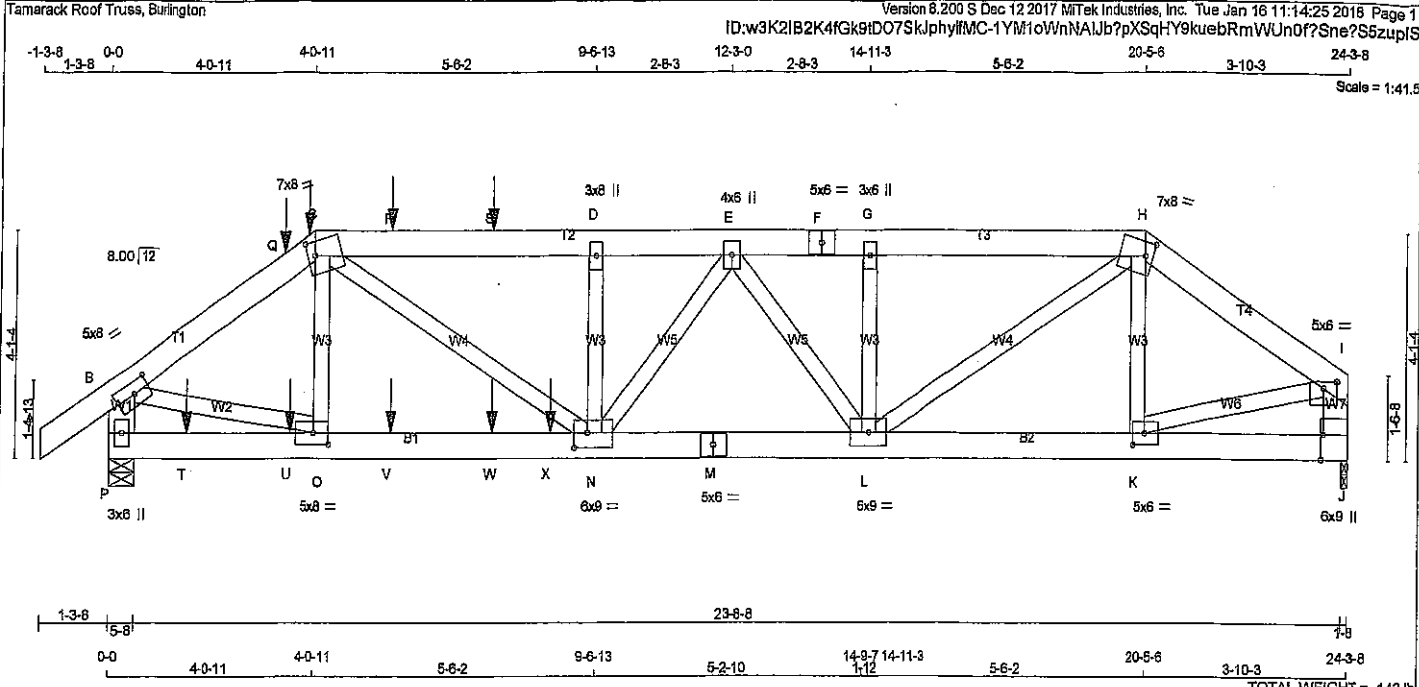
DATE	04/18/18
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 285749 LOCATION:
 BUILDER: GREENPARK - RUSSELL GARDENS PH '2 SUB-BUILDER:
 MODEL: HIGHGROVE 5E ELEVATION: 1(REV)- UNIT: 4

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
5	Hangers	LJS26DS	

TOTAL # ITEMS= 7.00



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	NO.2	DESCR.
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - I	2x6	DRY	No.2	SPF
P - B	2x6	DRY	No.2	SPF
J - I	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF

ALL WEBS 2x4 DRY No.2 SPF

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWV-I	MT20	5.0	8.0	2.50	3.75
C	TTWW-m	MT20	7.0	8.0	3.00	1.75
D	TMWV-w	MT20	3.0	6.0		
E	TMWV-I	MT20	4.0	6.0		
F	TS-I	MT20	5.0	6.0		
G	TMWV-w	MT20	3.0	6.0		
H	TTWW-m	MT20	7.0	8.0	3.00	1.75
I	TMWV-p	MT20	5.0	6.0	1.50	3.00
J	BMV1+p	MT20	6.0	9.0	5.50	0.50
K	BMWV-I	MT20	5.0	6.0	2.50	2.75
L	BMWV-w	MT20	6.0	9.0		
M	BS-I	MT20	5.0	6.0		
N	BMWV-w	MT20	6.0	9.0	3.25	3.00
O	BMWV-I	MT20	5.0	6.0	2.50	3.50
P	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 122.8 lbs FACTORED DOWN AT 3-8-12, 101.4 lbs FACTORED DOWN AT 5-6-12, AND 101.4 lbs FACTORED DOWN AT 7-6-12, AND 244.5 lbs FACTORED DOWN AT 4-0-11 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 1-6-12, 69.9 lbs FACTORED DOWN AT 3-6-12, 69.9 lbs FACTORED DOWN AT 5-6-12, AND 69.9 lbs FACTORED DOWN AT 7-6-12, AND 1245.8 lbs FACTORED DOWN AT 6-8-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	2308	1357 / 0	474 / 0	0 / 0
P	2868	0	2868	0
J	1954	0	1954	0

UNFACTORED REACTIONS

	1ST CASE	MAX/MIN COMPONENT REACTIONS		
	COMBINED	SNOW	LIVE	PERM.LIVE
JT	2308	1357 / 0	474 / 0	0 / 0
P	2868	0	2868	0
J	1954	0	1954	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.68 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. PERM. LIVE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. PERM. LIVE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO				FR-TO			
A-B	0 / 33	-84.3	-84.3 0.07 (1)	10.00	C-C	-135 / 261	0.05 (3)
B-Q	-3431 / 0	-84.3	-84.3 0.22 (1)	4.39	C-N	0 / 2219	0.39 (1)
Q-C	-3431 / 0	-84.3	-84.3 0.22 (1)	4.39	N-D	-584 / 0	0.10 (1)
C-R	-4718 / 0	-84.3	-84.3 0.45 (1)	3.58	L-G	-441 / 0	0.08 (1)
R-S	-4718 / 0	-84.3	-84.3 0.45 (1)	3.58	L-H	0 / 2162	0.38 (1)
S-D	-4718 / 0	-84.3	-84.3 0.45 (1)	3.58	K-H	-298 / 37	0.05 (1)
D-E	-4718 / 0	-84.3	-84.3 0.30 (1)	3.75	S-O	0 / 2934	0.52 (1)
E-F	-3683 / 0	-84.3	-84.3 0.20 (1)	4.28	K-I	0 / 1951	0.34 (1)
F-G	-3683 / 0	-84.3	-84.3 0.20 (1)	4.28	N-E	0 / 850	0.15 (1)
G-H	-3683 / 0	-84.3	-84.3 0.28 (1)	4.21	E-L	-896 / 0	0.21 (1)
H-I	-2276 / 0	-84.3	-84.3 0.14 (1)	5.17			
P-B	-2847 / 0	0.0	0.0 0.20 (1)	6.26			
J-I	-1822 / 0	0.0	0.0 0.14 (1)	7.22			
P-T	0 / 0	-28.0	-28.0 0.23 (2)	10.00			
T-U	0 / 0	-28.0	-28.0 0.23 (2)	10.00			
U-O	0 / 0	-28.0	-28.0 0.23 (2)	10.00			
O-V	0 / 2864	-28.0	-28.0 0.83 (1)	10.00			
V-W	0 / 2864	-28.0	-28.0 0.83 (1)	10.00			
W-X	0 / 2864	-28.0	-28.0 0.83 (1)	10.00			
X-N	0 / 2864	-28.0	-28.0 0.83 (1)	10.00			
N-M	0 / 4215	-28.0	-28.0 0.83 (1)	10.00			
M-L	0 / 4215	-28.0	-28.0 0.83 (1)	10.00			
L-K	0 / 1877	-28.0	-28.0 0.34 (1)	10.00			
K-J	0 / 0	-28.0	-28.0 0.10 (2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LOC1	MAX-	MAX+	FACE	DIR.	TYPE
C	4-0-11	-23	-25		FRONT	VERT	DEAD
C	4-0-11	-222	-222		FRONT	VERT	SNOW
Q	3-6-12	-123	-123		BACK	VERT	TOTAL
R	5-6-12	-101	-101		BACK	VERT	TOTAL
S	7-6-12	-101	-101		BACK	VERT	TOTAL
T	1-6-12	-40	-70		BACK	VERT	TOTAL
U	3-6-12	-40	-70		BACK	VERT	TOTAL
V	5-6-12	-40	-70		BACK	VERT	TOTAL
W	7-6-12	-40	-70		BACK	VERT	TOTAL
X	8-8-8	-1246	-1246		BACK	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***

GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.

LOADS WERE DERIVED FROM USER INPUT

NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***

ADDD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/380 (0.81")$

CALCULATED VERT. DEFL.(LL) = $L/999 (0.15")$

ALLOWABLE DEFL.(TL) = $L/380 (0.81")$

CALCULATED VERT. DEFL.(TL) = $L/999 (0.24")$

CSI: TC=0.45/1.00 (C-D-1), BC=0.83/1.00 (L-N-1), WB=0.52/1.00 (B-O-1), SS=0.78/1.00 (N-O-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00

COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

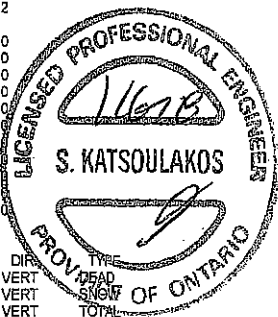
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)
MT20	618	354	1667 822 2284 1656

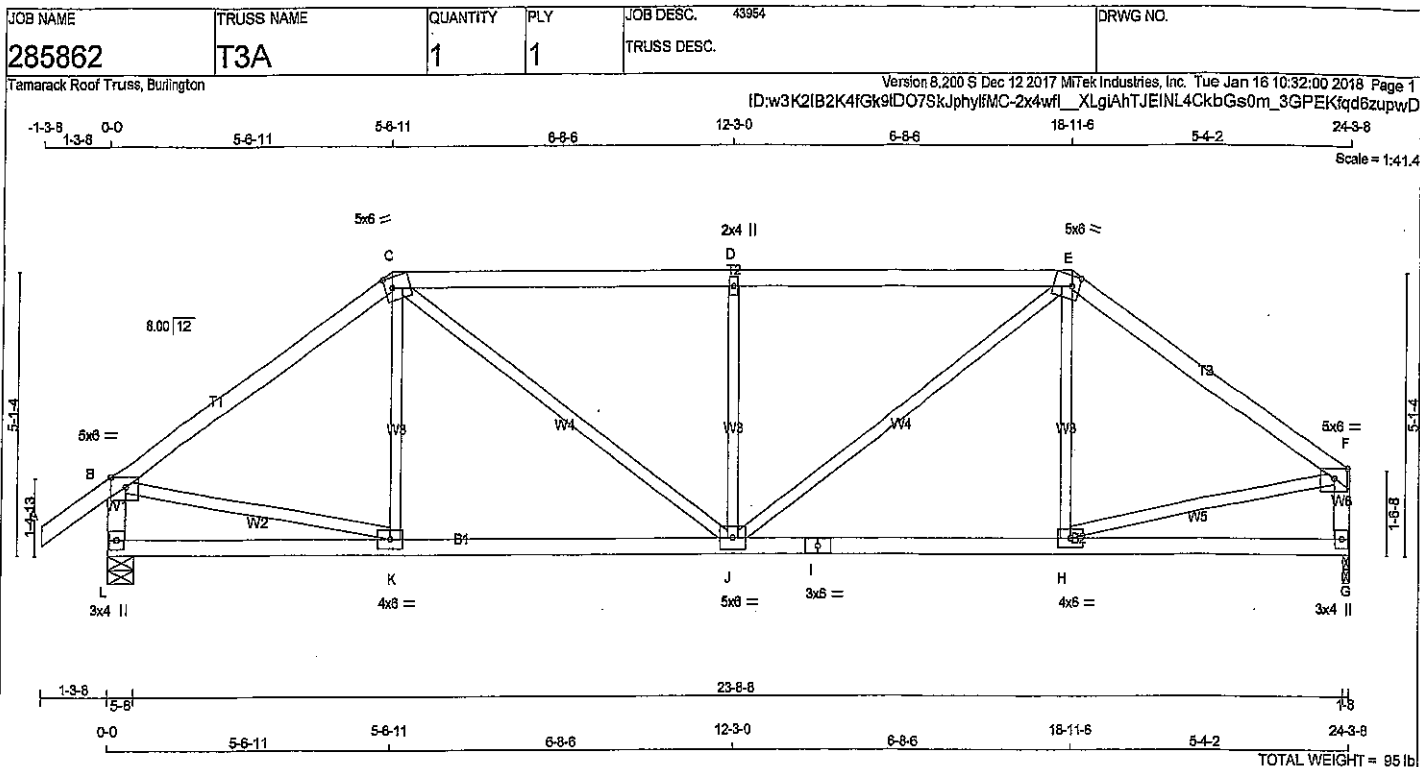
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.90 (B) (INPUT = 0.90)

JSI METAL=0.90 (M) (INPUT = 1.00)





LUMBER

N.L.G.A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
L - B	2x4	DRY	No.2
G - F	2x4	DRY	No.2
L - I	2x4	DRY	No.2
I - G	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW-m	MT20	5.0	8.0	2.25	1.75
D	TMVW-w	MT20	2.0	4.0		
E	TTWW-m	MT20	5.0	8.0	2.25	1.75
F	TMVW-p	MT20	5.0	8.0	Edge	
G	BMV1+p	MT20	3.0	4.0		
H	BMVW-t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	8.0		
J	BMVW-t	MT20	5.0	6.0		
K	BMVW-t	MT20	4.0	8.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
L	1479	0	1479	0	5-8	5-8
G	1363	0	1363	0	1-8	1-8

UNFACTORED REACTIONS

JT	MAX./MIN. COMPONENT REACTIONS						
	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1198	682 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0
G	1119	821 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

FR-TO	CHORDS		FACTORED		WEBS		FACTORED	
	MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX	MEMB.	FORCE (LBS)	MAX	LC1 (C)
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	K-C	-45 / 211	0.05 (3)
B-C	-1495 / 0	-84.3	-84.3	0.55 (1)	4.69	C-J	0 / 728	0.16 (1)
C-D	-1833 / 0	-84.3	-84.3	0.80 (1)	3.88	J-D	-892 / 0	0.27 (1)
D-E	-1833 / 0	-84.3	-84.3	0.80 (1)	3.88	J-E	0 / 769	0.17 (1)
E-F	-1456 / 0	-84.3	-84.3	0.50 (1)	4.81	H-E	-81 / 167	0.04 (3)
L-B	-1418 / 0	0.0	0.0	0.15 (1)	6.85	B-K	0 / 1265	0.28 (1)
G-F	-1305 / 0	0.0	0.0	0.14 (1)	7.06	H-F	0 / 1239	0.28 (1)
L-K	0 / 0	-28.0	-28.0	0.25 (3)	10.00			
K-J	0 / 1241	-28.0	-28.0	0.40 (2)	10.00			
J-I	0 / 1208	-28.0	-28.0	0.39 (2)	10.00			
I-H	0 / 1208	-28.0	-28.0	0.39 (2)	10.00			
H-G	0 / 0	-28.0	-28.0	0.24 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/999 (0.14")

CSI: TC=0.80/1.00 (D-E:1), BC=0.40/1.00 (J-K:2), WB=0.28/1.00 (B-K:1), SS=0.27/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1867 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (E) (INPUT = 0.90)
JSI METAL= 0.36 (I) (INPUT = 1.00)



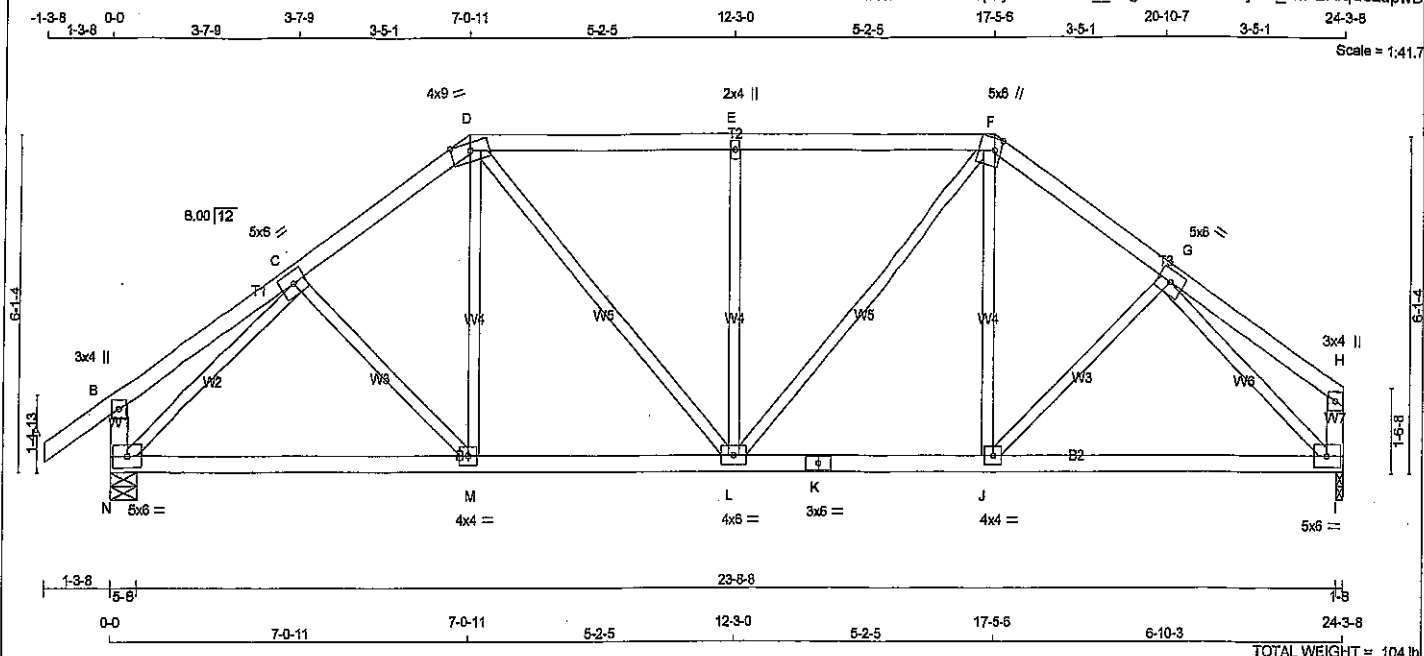
DRWG NO. TAM 4008-18
STRUCTURAL
COMPONENT ONLY

JOB NAME 285862	TRUSS NAME T4A	QUANTITY 1	PLY 1	JOB DESC. 43854	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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LUMBER			LUMBER			DESCR.		
N. L. G. A. RULES								
CHORDS	SIZE							
A - D	2x4	DRY	No.2	SPF				
D - F	2x4	DRY	No.2	SPF				
F - H	2x4	DRY	No.2	SPF				
N - B	2x4	DRY	No.2	SPF				
I - H	2x4	DRY	No.2	SPF				
N - K	2x4	DRY	No.2	SPF				
K - I	2x4	DRY	No.2	SPF				

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

ORY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW-m	MT20	4.0	9.0	Edge	
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.50	1.50
G	TMWW-t	MT20	5.0	6.0		
H	TMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	5.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMVW1-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORIZ	DOWN	HORIZ
N	1479	0	1479	0
I	1363	0	1363	0

UNFACTORED REACTIONS

1ST CASE	MAX/MIN	COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1198	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0
I	1119	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.05 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS! (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS! (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 32	-84.3 -84.3	0.11 (1)	C-M	0 / 117	0.03 (3)	
B-C	0 / 17	-84.3 -84.3	0.15 (1)	M-D	0 / 305	0.07 (2)	
C-D	-1459 / 0	-84.3 -84.3	0.14 (1)	D-L	0 / 409	0.09 (1)	
D-E	-1474 / 0	-84.3 -84.3	0.31 (1)	L-E	-534 / 0	0.31 (1)	
E-F	-1474 / 0	-84.3 -84.3	0.31 (1)	L-F	0 / 442	0.10 (1)	
F-G	-1433 / 0	-84.3 -84.3	0.14 (1)	J-F	0 / 273	0.06 (2)	
G-H	0 / 17	-84.3 -84.3	0.14 (1)	J-G	0 / 138	0.03 (3)	
N-B	-233 / 0	0.0 0.0	0.02 (1)	N-C	-1669 / 0	0.64 (1)	
I-H	-108 / 0	0.0 0.0	0.01 (1)	G-I	-1635 / 0	0.59 (1)	
N-M	0 / 1196	-28.0 -28.0	0.45 (2)				
M-L	0 / 1201	-28.0 -28.0	0.46 (2)				
L-K	0 / 1179	-28.0 -28.0	0.45 (2)				
K-J	0 / 1179	-28.0 -28.0	0.45 (2)				
J-I	0 / 1136	-28.0 -28.0	0.43 (2)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 3.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 46.1	PSF

SPACING = 24.0 IN. GIC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/999 (0.18")

CS! TC=0.31/1.00 (D-E:1), BC=0.46/1.00 (L-M:2), WB=0.64/1.00 (C-N:1), SS! =0.21/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

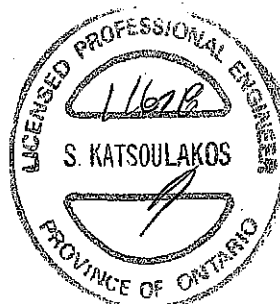
NAIL VALUES

PLATE GRIP(DRY) SHEAR	SECTION
(PSI)	(PLI)
MAX MIN	MAX MIN
MT20	618 354 1687 622 2284 1658

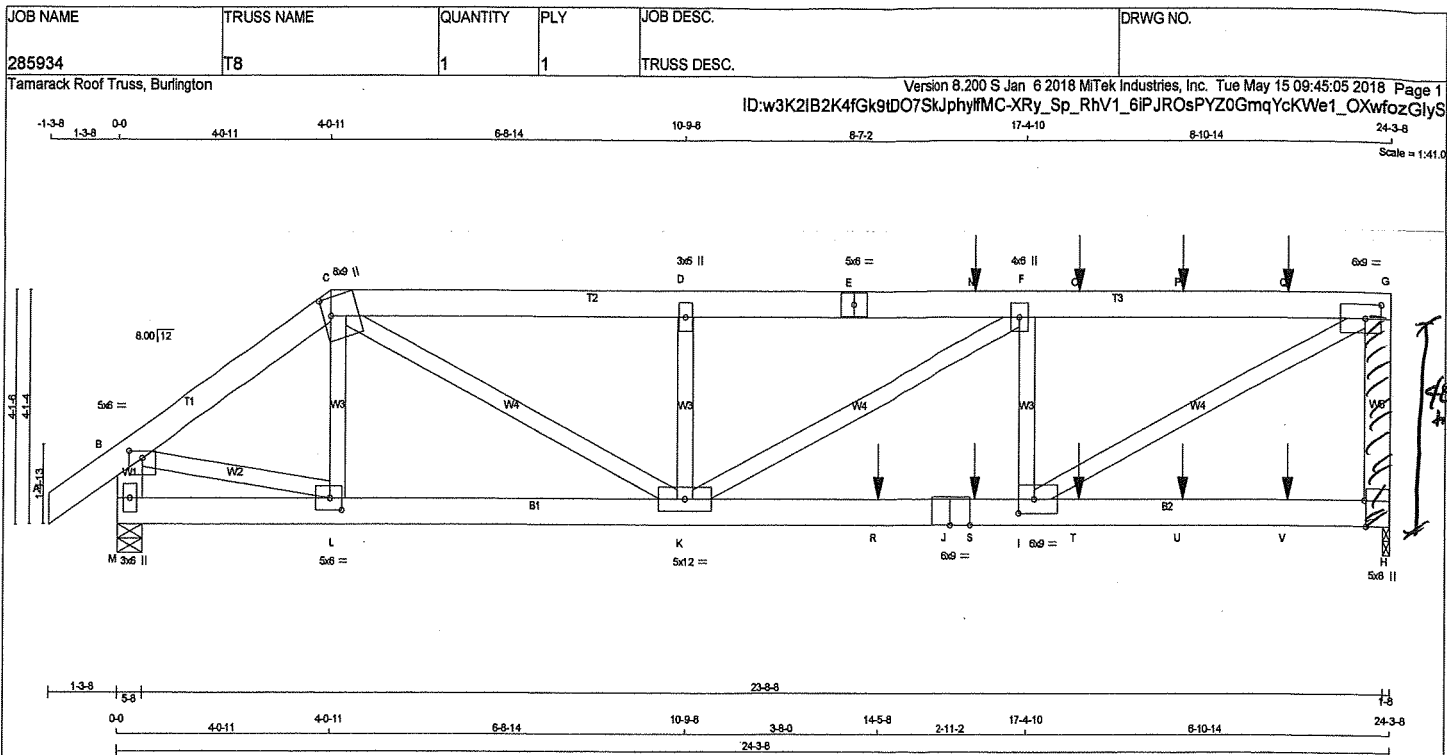
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (D) (INPUT = 0.90)
JSI METAL= 0.41 (C) (INPUT = 1.00)



DRWG NO. TAM 4009-18
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 138 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - G	2x6	DRY	No.2	SPF
H - G	2x6	DRY	No.2	SPF
M - B	2x6	DRY	No.2	SPF
M - J	2x6	DRY	2100F 1.8E	SPF
J - H	2x6	DRY	2100F 1.8E	SPF

ALL WEBS 2x4 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TTVW+m	MT20	8.0	9.0	3.75	1.75
D	TMVW+w	MT20	3.0	6.0		
E	TS-t	MT20	5.0	6.0		
F	TMVW+t	MT20	4.0	6.0		
G	TMVW-t	MT20	6.0	9.0	3.00	3.50
H	BMV1+p	MT20	5.0	8.0	Edge	0.25
I	BMVW-t	MT20	6.0	9.0	3.00	3.50
J	BS-t	MT20	6.0	9.0		
K	BMVW-t	MT20	5.0	12.0		
L	BMVW-t	MT20	5.0	6.0	2.50	2.75
M	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	UPLIFT	IN-SX
H	2719	0	0	1-8
M	2212	0	0	5-8

UNFACTORED REACTIONS

	1ST CASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
H	2211	1262 / 0	480 / 0	0 / 0	0 / 0	468 / 0	0 / 0
M	1781	1046 / 0	367 / 0	0 / 0	0 / 0	368 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.50 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE	VERT. LOAD LC1	MEMB.	FORCE
(LBS)	(LBS)	(LBS)	(LBS)	(LBS)
FR-TO		FROM TO	FR-TO	
A-B	0 / 33	-84.3 -84.3 0.07 (1)	L-C	-437 / 17
B-C	-2504 / 0	-84.3 -84.3 0.17 (1)	B-L	0 / 2124
C-D	-4685 / 0	-84.3 -84.3 0.45 (1)	I-G	0 / 5007
D-E	-4685 / 0	-84.3 -84.3 0.55 (1)	C-K	0 / 2958
E-N	-4685 / 0	-84.3 -84.3 0.55 (1)	I-F	-1063 / 0
N-F	-4685 / 0	-84.3 -84.3 0.55 (1)	K-D	-564 / 0
F-O	-4428 / 0	-84.3 -84.3 0.61 (1)	K-F	0 / 272
O-P	-4428 / 0	-84.3 -84.3 0.61 (1)		
P-Q	-4428 / 0	-84.3 -84.3 0.61 (1)		
Q-G	-4428 / 0	-84.3 -84.3 0.61 (1)		
H-G	-2691 / 0	0.0 0.0 0.39 (1)		
M-B	-2131 / 0	0.0 0.0 0.15 (1)		

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE	VERT. LOAD LC1	MEMB.	FORCE
(LBS)	(LBS)	(LBS)	(LBS)	(LBS)
M-L	0 / 0	-28.0 -28.0 0.05 (1)		
L-K	0 / 2060	-28.0 -28.0 0.39 (1)		
K-R	0 / 4428	-28.0 -28.0 0.98 (1)		
R-J	0 / 4428	-28.0 -28.0 0.98 (1)		
J-S	0 / 4428	-28.0 -28.0 0.98 (1)		
S-I	0 / 4428	-28.0 -28.0 0.98 (1)		
H-I	0 / 0	-28.0 -28.0 0.33 (1)		
T-U	0 / 0	-28.0 -28.0 0.33 (1)		
U-V	0 / 0	-28.0 -28.0 0.33 (1)		
V-H	0 / 0	-28.0 -28.0 0.33 (1)		

SPECIFIED CONCENTRATED LOADS (LBS)

J	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TOTAL
N	18-4-12	-69	-69		BACK	VERT	TOTAL
O	18-4-12	-69	-69		BACK	VERT	TOTAL
P	20-4-12	-69	-69		BACK	VERT	TOTAL
Q	22-4-12	-69	-69		BACK	VERT	TOTAL
R	14-5-8	-1197	-1197		BACK	VERT	TOTAL
S	18-4-12	-50	-50		BACK	VERT	TOTAL
T	18-4-12	-50	-50		BACK	VERT	TOTAL
U	20-4-12	-50	-50		BACK	VERT	TOTAL
V	22-4-12	-50	-50		BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
	DL = 3.0 PSF
BOT CH.	LL = 10.5 PSF
	DL = 7.0 PSF
TOTAL LOAD	= 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/968 (0.30")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/615 (0.47")

CSI: TC=0.61/1.00 (F-G:1), BC=0.98/1.00 (I-K:1),
WB=0.89/1.00 (G-I:1), SS=0.68/1.00 (I-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR	SECTION
(PSI)	(PLI)
MAX MIN MAX MIN MAX MIN	
MT20	618 354 1667 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.90 (L) (INPUT = 0.90)
JSI METAL= 0.91 (J) (INPUT = 1.00)

CONTINUED ON PAGE 2

DWG NO. TAM 21/54-18 R1
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285934	T8	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

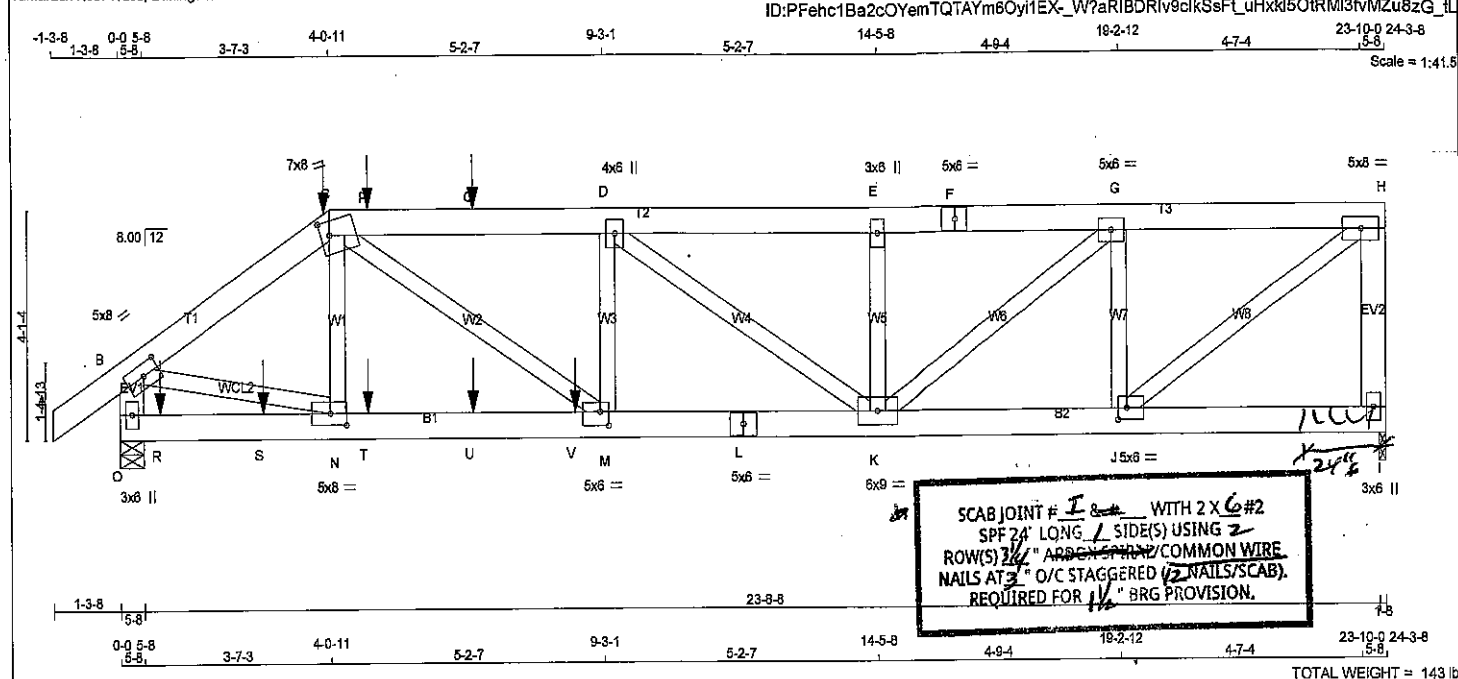
Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue May 15 09:45:05 2018 Page 2
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HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 101.4 lbs FACTORED DOWN AT
16-4-12, 101.4 lbs FACTORED DOWN AT
18-4-12, AND 101.4 lbs FACTORED DOWN AT
20-4-12, AND 101.4 lbs FACTORED DOWN AT
22-4-12 ON TOP CHORD, AND 1519.0 lbs
FACTORED DOWN AT 14-5-8, 69.9 lbs
FACTORED DOWN AT 16-4-12, 69.9 lbs
FACTORED DOWN AT 18-4-12, AND 69.9 lbs
FACTORED DOWN AT 20-4-12, AND 69.9 lbs
FACTORED DOWN AT 22-4-12 ON BOTTOM
CHORD. DESIGN FOR UNSPECIFIED
CONNECTION(S) IS DELEGATED TO THE
BUILDING DESIGNER.



for

DWG NO. TAM 21154 -18 R1
STRUCTURAL
COMPONENT ONLY



<u>LUMBER</u>				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
I - H	2x6	DRY	No.2	SPF
O - B	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF
EXCEPT				
DRY, SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMWV-t	MT20	5.0	8.0	2.50 3.75
C	TTWW-m	MT20	7.0	8.0	3.00 2.00
D	TMWV-H	MT20	4.0	8.0	
E	TMW-w	MT20	3.0	6.0	
F	TS-t	MT20	5.0	6.0	
G	TMWV-t	MT20	5.0	6.0	
H	TMWV-I	MT20	5.0	8.0	
I	BMV1+p	MT20	3.0	8.0	
J	BMWV-t	MT20	5.0	6.0	2.50 2.00
K	BMWWVW-I	MT20	6.0	9.0	
L	BS-t	MT20	5.0	6.0	
M	BMWV-I	MT20	5.0	8.0	3.00 2.00
N	BMWV-t	MT20	5.0	8.0	2.50 3.75
O	BMV1+p	MT20	3.0	6.0	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 244.5 lbs FACTORED DOWN AT 4-0-11, AND 113.4 lbs FACTORED DOWN AT 4-9-4, AND 101.4 lbs FACTORED DOWN AT 6-9-4 ON TOP CHORD, AND 75.5 lbs FACTORED DOWN AT 8-4, 69.9 lbs FACTORED DOWN AT 2-8-4, 89.9 lbs FACTORED DOWN AT 4-9-4, AND 69.9 lbs FACTORED DOWN AT 6-9-4, AND 119.0 lbs FACTORED DOWN AT 8-8-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REORD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT IN-SX	IN-SX
I	1894	0	1894	0	0	1-8
O	2731	0	2731	0	0	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1548	871 / 0	345 / 0	0 / 0	0 / 0	332 / 0	0 / 0
O	2724	1265 / 0	486 / 0	0 / 0	0 / 0	473 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.73 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY
APPLIED

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

LOADING
TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MAX. FACTORED		FACTORED			MAX. FACTORED				
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRAC	MEMB.	FORCE (LBS)	MAX CSI (LC)		
FR-TO		FROM TO	CSI (LC)	LENGTH	FR-TO				
A-B	0/33	-84.3	-84.3	0.07 (1)	10.00	N-C	-170/231	0.04 (3)	
B-C	-3245/0	-84.3	-84.3	0.19 (1)	4.53	C-M	0/2150	0.38 (1)	
C-P	-4458/0	-84.3	-84.3	0.41 (1)	3.73	M-D	0/237	0.04 (3)	
P-Q	-4458/0	-84.3	-84.3	0.41 (1)	3.73	D-K	-1017/0	0.49 (1)	
Q-D	-4458/0	-84.3	-84.3	0.41 (1)	3.73	K-E	-375/0	0.06 (1)	
D-E	-3829/0	-84.3	-84.3	0.26 (1)	4.27	K-G	0/1673	0.30 (1)	
E-F	-3829/0	-84.3	-84.3	0.22 (1)	4.30	J-G	-1482/0	0.25 (1)	
F-G	-3829/0	-84.3	-84.3	0.22 (1)	4.30	J-H	0/2633	0.50 (1)	
G-H	-2300/0	-84.3	-84.3	0.18 (1)	5.21	B-N	0/2753	0.49 (1)	
I-H	-1833/0	0.0	0.0	0.27 (1)	7.35				
O-B	-2875/0	0.0	0.0	0.19 (1)	6.33				
O-R	0/0	-28.0	-28.0	0.19 (2)	10.00				
R-S	0/0	-28.0	-28.0	0.19 (2)	10.00				
S-N	0/0	-28.0	-28.0	0.19 (2)	10.00				
N-T	0/2686	-28.0	-28.0	0.67 (1)	10.00				
T-U	0/2686	-28.0	-28.0	0.67 (1)	10.00				
U-V	0/2686	-28.0	-28.0	0.67 (1)	10.00				
V-M	0/2686	-28.0	-28.0	0.67 (1)	10.00				
M-L	0/4458	-28.0	-28.0	0.77 (1)	10.00				
L-K	0/4458	-28.0	-28.0	0.77 (1)	10.00				
K-J	0/2300	-28.0	-28.0	0.37 (1)	10.00				
J-I	0/0	-28.0	-28.0	0.08 (3)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX	MAX*	FACE	DIR.	TYPE
P	4-0-11	-23	-25	---	FRONT	VERT	DEAD
C	4-0-11	-222	-222	---	FRONT	VERT	SNOW
O	4-9-4	-113	-113	---	FRONT	VERT	TOTAL
Q	6-8-4	-101	-101	---	FRONT	VERT	TOTAL
R	9-4	-43	-76	---	FRONT	VERT	TOTAL
S	2-6-4	-40	-70	---	FRONT	VERT	TOTAL
T	4-9-4	-40	-70	---	FRONT	VERT	TOTAL
U	6-9-4	-40	-70	---	FRONT	VERT	TOTAL
V	8-8-8	-1157	-1157	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED
BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 8.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO
ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= $L/360$ (0.81")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.14")
ALLOWABLE DEFL.(TL)= $L/360$ (0.81")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.22")

CSI: TC=0.41/1.00 (C-D:1), BC=0.77/1.00 (K-M:1),
WB=0.50/1.00 (H-I:1), SSI=0.74/1.00 (M-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP(DRY)		SHEAR		SECTION
	(PSI)		(PLI)		(PLI)
	MAX	MIN	MAX	MIN	MAX MIN
MT20	818	354	1887	822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

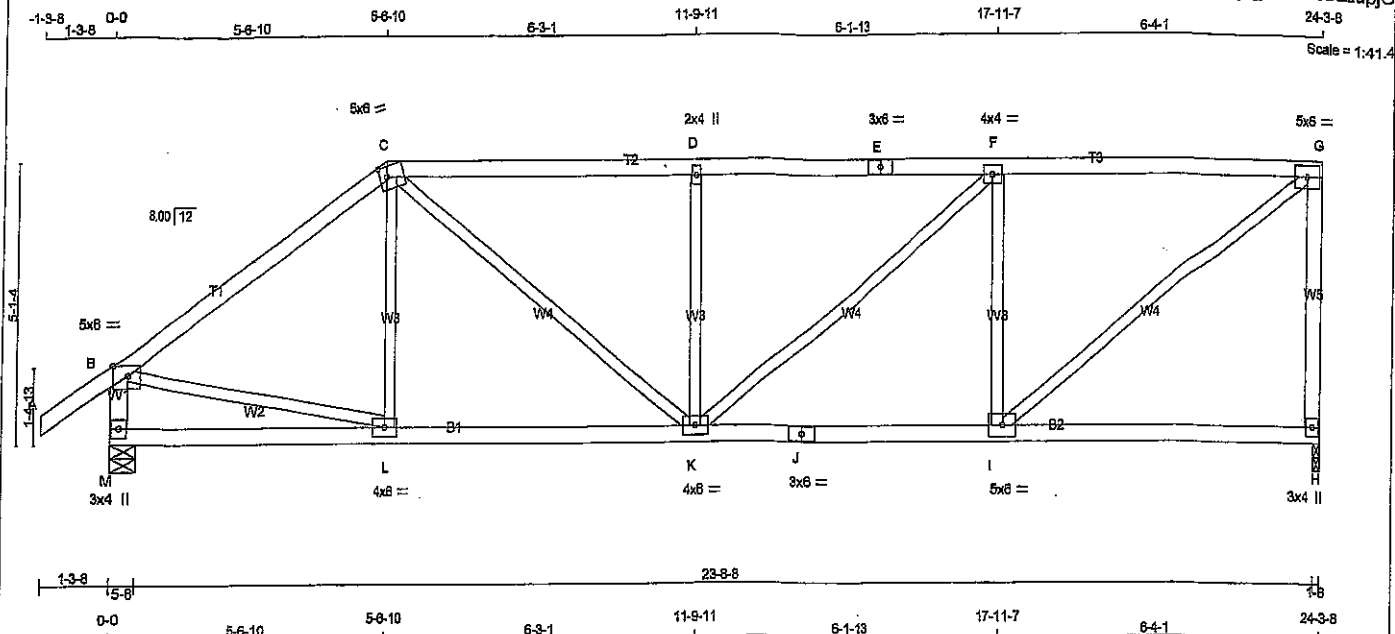
JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.66 (L) (INPUT = 1.00)

DWG NO. TAM 26863-78
 STRUCTURAL
 COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43854	DRWG NO.
285994	T9	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER	SIZE	NUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
H - J	2x4	DRY	No.2
M - B	2x4	DRY	No.2
M - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE				
B TMVW-p	MT20	5.0	6.0	Edge
C TTWW-m	MT20	5.0	6.0	2.25 1.75
D TMW-w	MT20	2.0	4.0	
E TS-t	MT20	3.0	6.0	
F TMVW-t	MT20	4.0	4.0	
G TMVW-t	MT20	5.0	6.0	
H BMV1+p	MT20	3.0	4.0	
I BMVW-t	MT20	5.0	6.0	
J BS-t	MT20	3.0	6.0	
K BMVWV-t	MT20	4.0	6.0	
L BMVW-t	MT20	4.0	6.0	
M BMV1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	HORIZ	UPLIFT
H	1353	0	1363	0
M	1479	0	1479	0

UNFACTORED REACTIONS	1ST LCASE	MAX/MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
H	1119	621 / 0 255 / 0 0 / 0 0 / 0 243 / 0 0 / 0
M	1198	692 / 0 255 / 0 0 / 0 0 / 0 251 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.34 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)
FR-TO									
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	L-C	-49 / 204	0.05 (3)	
B-C	-1493 / 0	-84.3	-84.3	0.55 (1)	4.89	B-L	0 / 1263	0.28 (1)	
C-D	-1797 / 0	-84.3	-84.3	0.55 (1)	4.37	I-G	0 / 1778	0.40 (1)	
D-E	-1797 / 0	-84.3	-84.3	0.56 (1)	4.34	C-K	0 / 702	0.16 (1)	
E-F	-1797 / 0	-84.3	-84.3	0.56 (1)	4.34	I-F	-877 / 0	0.34 (1)	
F-G	-1415 / 0	-84.3	-84.3	0.53 (1)	4.81	K-D	-566 / 0	0.22 (1)	
H-G	-1282 / 0	0.0	0.0	0.57 (1)	7.10	K-F	0 / 484	0.11 (1)	
M-B	-1417 / 0	0.0	0.0	0.15 (1)	6.85				
M-L	0 / 0	-28.0	-28.0	0.24 (3)	10.00				
L-K	0 / 1240	-28.0	-28.0	0.39 (2)	10.00				
K-J	0 / 1418	-28.0	-28.0	0.44 (2)	10.00				
J-I	0 / 1416	-28.0	-28.0	0.44 (2)	10.00				
I-H	0 / 0	-28.0	-28.0	0.27 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/999 (0.09")
ALLOWABLE DEFL.(TL) = L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/999 (0.14")

CSI: TC=0.57/1.00 (G-H:1), BC=0.44/1.00 (I-K:2),
WB=0.40/1.00 (G-I:1), CSI=0.25/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

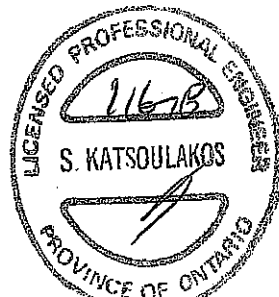
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1887 822 2284 1655

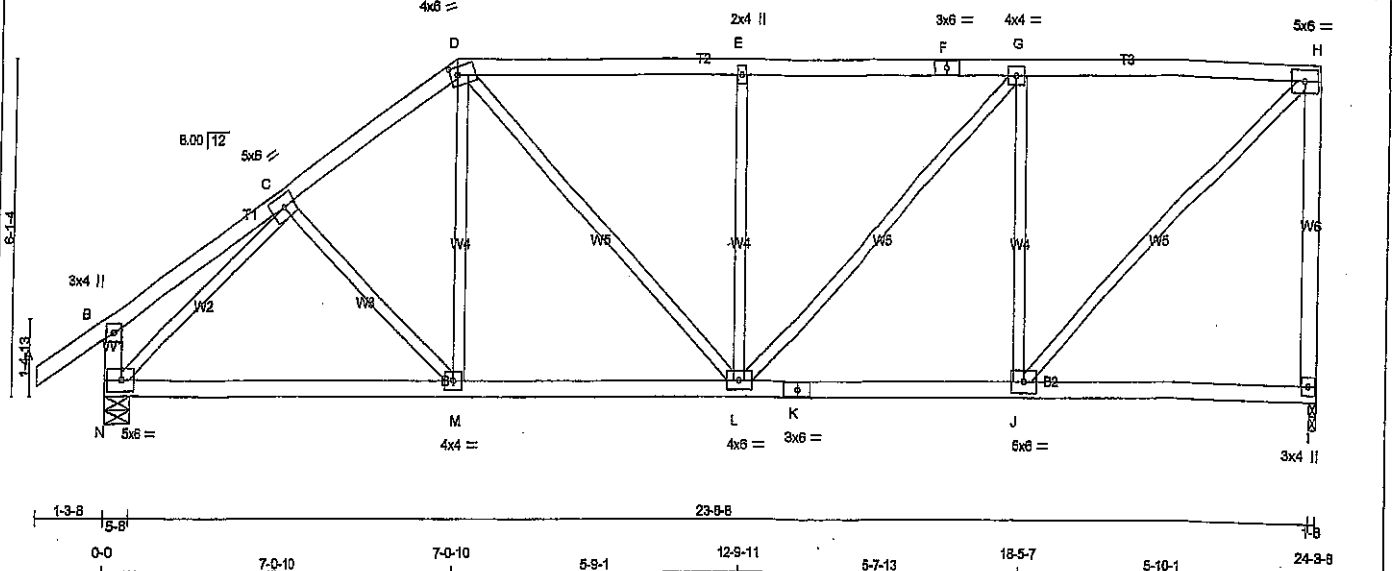
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (K) (INPUT = 0.80)
JSI METAL= 0.44 (J) (INPUT = 1.00)



DRWG NO. TAM 3988-18
STRUCTURAL
COMPONENT ONLY



LUMBER

N.L.G.A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
I - H	2x4	DRY	No.2
N - B	2x4	DRY	No.2
N - K	2x4	DRY	No.2
K - I	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW+t	MT20	5.0	8.0		
D	TTWW+m	MT20	4.0	6.0	1.75	1.75
E	TMV+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMVW+t	MT20	4.0	4.0		
H	TMVW-t	MT20	5.0	6.0		
I	BMV1+p	MT20	3.0	4.0		
J	BMVW-t	MT20	5.0	6.0		
K	BS-t	MT20	3.0	8.0		
L	BMVW+t	MT20	4.0	6.0		
M	BMVW-t	MT20	4.0	4.0		
N	BMVW-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED		MAXIMUM FACTORED		INPUT	RECORD
	GROSS REACTION	GROSS REACTION	GROSS REACTION	GROSS REACTION		
I	1393	0	1363	0	1-8	1-8
N	1479	0	1479	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
I	1119	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
N	1198	892 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.85 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO				FR-TO			
A-B	0 / 32	-84.3	-84.3 0.11 (1)	10.00	C-M	0 / 118	0.03 (3)
B-C	0 / 17	-84.3	-84.3 0.15 (1)	10.00	M-D	0 / 307	0.07 (2)
C-D	-1460 / 0	-84.3	-84.3 0.18 (1)	5.22	N-C	-1669 / 0	0.84 (1)
D-E	-1473 / 0	-84.3	-84.3 0.43 (1)	4.88	J-H	0 / 1557	0.35 (1)
E-F	-1473 / 0	-84.3	-84.3 0.44 (1)	4.88	D-L	0 / 385	0.09 (1)
F-G	-1473 / 0	-84.3	-84.3 0.44 (1)	4.88	J-G	-912 / 0	0.53 (1)
G-H	-1103 / 0	-84.3	-84.3 0.42 (1)	5.46	L-E	-520 / 0	0.30 (1)
I-H	-1298 / 0	0.0	0.0 0.94 (1)	7.08	L-G	0 / 530	0.12 (1)
N-B	-233 / 0	0.0	0.0 0.02 (1)	7.81			
N-M	0 / 1198	-28.0	-28.0 0.45 (2)	10.00			
M-L	0 / 1202	-28.0	-28.0 0.47 (2)	10.00			
L-K	0 / 1103	-28.0	-28.0 0.37 (2)	10.00			
K-J	0 / 1103	-28.0	-28.0 0.37 (2)	10.00			
J-I	0 / 0	-28.0	-28.0 0.24 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN PLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL) = L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/999 (0.19")

CS1: TC=0.94/1.00 (H=1), BC=0.47/1.00 (L=M=2), WB=0.64/1.00 (C=N=1), SSI=0.23/1.00 (G=H=1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

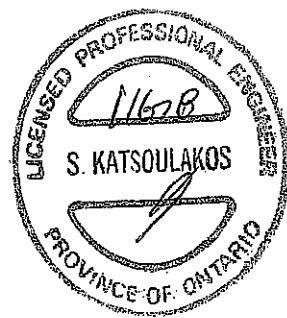
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.80 (D) (INPUT = 0.80)
JSI METAL= 0.41 (C) (INPUT = 1.00)

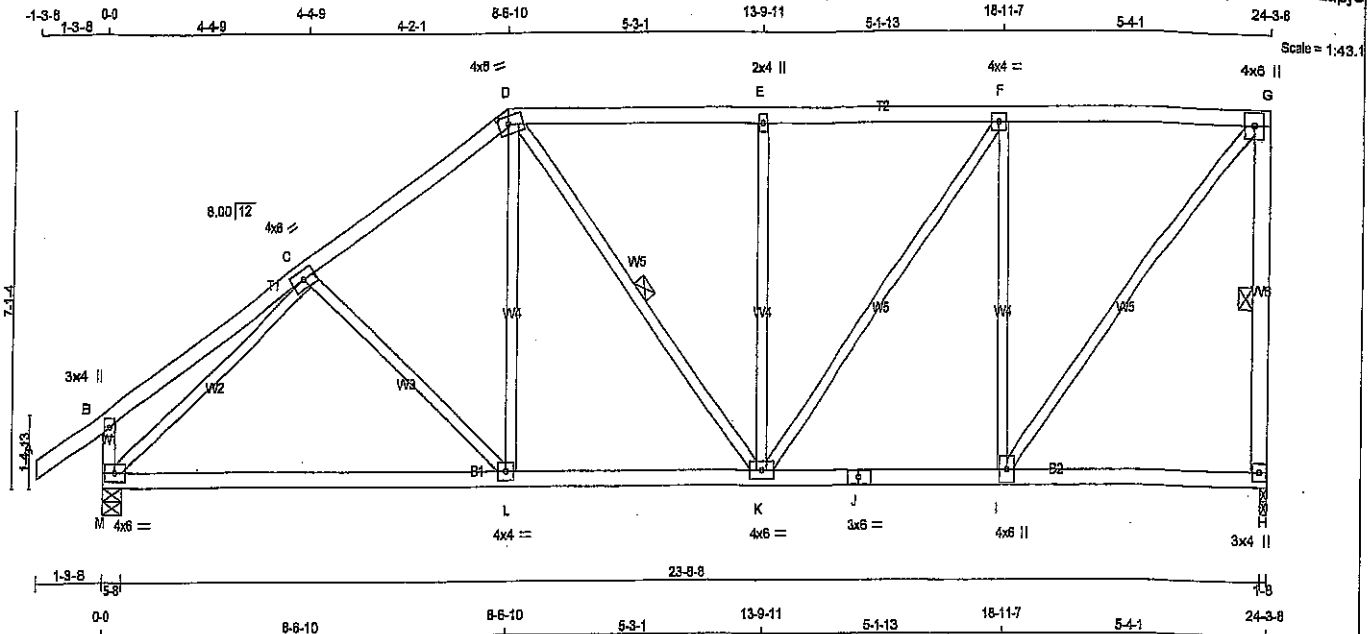


DRWG NO. TAM 3989-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
285994	T11	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.200 S Dec 12 2017 Mitek Industries, Inc. Tue Jan 16 10:45:49 2018 Page 1
ID:w3K2IB2K4fGk9tDO7SkJphyfMC-HQgJemO_136475LelukB0aN21vgjqMQsozCDZupjG



TOTAL WEIGHT = 2 X 113 = 226 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - G	2x4	DRY	No.2	SPF	
H - G	2x4	DRY	No.2	SPF	
M - B	2x4	DRY	No.2	SPF	
M - J	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	4.0	6.0		
D	TTVW-m	MT20	4.0	8.0	1.75	2.50
E	TMV+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	4.0		
G	TMVW+p	MT20	4.0	6.0		
H	BMV-t	MT20	3.0	4.0		
I	BMVW-t	MT20	4.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	SMVW-t	MT20	4.0	6.0		
L	SMVW-t	MT20	4.0	4.0		
M	BMVW-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	UP	IN-SX
H	1353	0	0	1-8
M	1478	0	0	5-8

UNFACTORED REACTIONS

JT	1ST CASE	MAX	MIN	COMPONENT REACTIONS
	COMBINED	SNOW	LIVE	PERM. LIVE
H	1118	821 / 0	255 / 0	0 / 0
M	1198	892 / 0	255 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-H, D-K

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (L)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM	TO		FR-TO			
A-B	0/32	-84.3	-84.3	0.11 (1)	10.00	L-L	-117/85	0.07 (1)
B-C	0/23	-84.3	-84.3	0.24 (1)	10.00	L-D	0/414	0.09 (2)
C-D	-1360/0	-84.3	-84.3	0.28 (1)	5.20	M-C	-1681/0	0.93 (1)
D-E	-1226/0	-84.3	-84.3	0.34 (1)	5.37	I-G	0/1426	0.32 (1)
E-F	-1227/0	-84.3	-84.3	0.35 (1)	5.35	D-K	-6/141	0.03 (1)
F-G	-879/0	-84.3	-84.3	0.34 (1)	6.07	I-F	-948/0	0.83 (1)
G-H	-1305/0	0.0	0.0	0.30 (1)	5.66	K-E	-474/0	0.41 (1)
H-I	-255/0	0.0	0.0	0.03 (1)	7.81	K-F	0/576	0.13 (1)
M-L	0/1223	-28.0	-28.0	0.59 (2)	10.00			
L-K	0/1141	-28.0	-28.0	0.59 (2)	10.00			
K-J	0/879	-28.0	-28.0	0.31 (2)	10.00			
J-I	0/879	-28.0	-28.0	0.31 (2)	10.00			
I-H	0/0	-28.0	-28.0	0.21 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD	=	46.1	PSF	

SPACING = 24.0 IN./G/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA C86-09
- TPIC 2011

(65 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/380$ (0.81")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.23")
ALLOWABLE DEFL.(TL) = $L/360$ (0.81")
CALCULATED VERT. DEFL.(TL) = $L/779$ (0.37")

CSI: TC=0.35/1.00 (E-F:1), BC=0.59/1.00 (K-L:2), WB=0.93/1.00 (C-M:1), SS=0.21/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

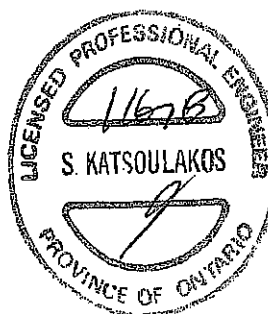
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	618	354	1667
	822	2284	1656

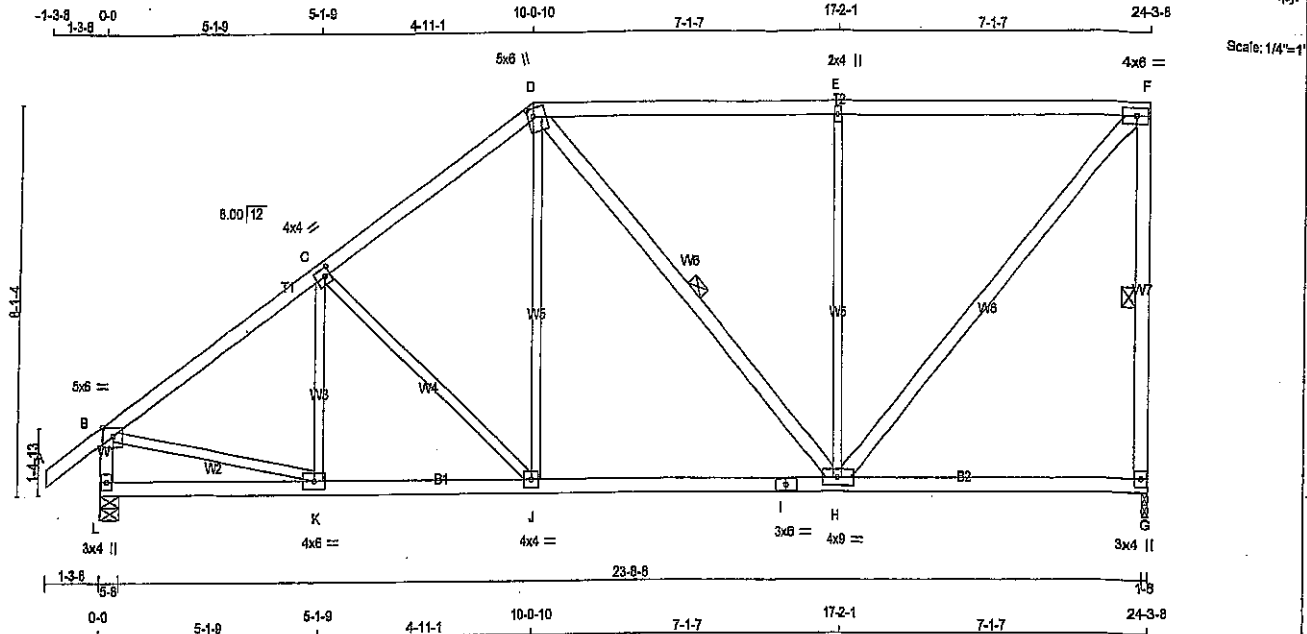
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (M) (INPUT = 0.90)
JSI METAL= 0.40 (C) (INPUT = 1.00)



DRWG NO. TAM 3990-18
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 2 X 117 = 235 lb

LUMBER

N.L.G.A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
G - B	2x4	DRY	No.2
L - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
I - G	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
D - H	2x4	DRY	No.2
H - F	2x4	DRY	No.2

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	UP/LIFT		
G	1383	0	1383	0	1-8	1-8
L	1479	0	1479	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LOASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
G	1110	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
L	1198	892 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.81 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LBS)
FR-TO		FROM TO			FR-TO		
A-B	0 / 32	-84.3 -84.3	0.11 (1)	10.00	K-C	-123 / 133	0.04 (1)
B-C	-1529 / 0	-84.3 -84.3	0.43 (1)	4.83	C-J	-347 / 0	0.29 (1)
C-D	-1275 / 0	-84.3 -84.3	0.40 (1)	5.20	J-D	0 / 436	0.10 (2)
D-E	-867 / 0	-84.3 -84.3	0.80 (1)	4.81	D-H	-110 / 0	0.06 (1)
E-F	-867 / 0	-84.3 -84.3	0.81 (1)	4.81	H-F	-745 / 0	0.84 (1)
G-F	-1284 / 0	0.0 0.0	0.38 (1)	5.69	H-F	0 / 1432	0.23 (1)
L-B	-1418 / 0	0.0 0.0	0.15 (1)	6.85	B-K	0 / 1323	0.30 (1)
L-K	0 / 0	-26.0 -26.0	0.18 (3)	10.00			
K-J	0 / 1295	-26.0 -26.0	0.33 (2)	10.00			
J-I	0 / 1041	-26.0 -26.0	0.49 (2)	10.00			
I-H	0 / 1041	-26.0 -26.0	0.49 (2)	10.00			
H-G	0 / 0	-26.0 -26.0	0.37 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(66 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/999 (0.10")
ALLOWABLE DEFL.(TL) = L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/999 (0.18")

CSI: TC=0.81/1.00 (E-F:1), BC=0.49/1.00 (H-J:2), WB=0.94/1.00 (E-H:1), SI=0.29/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

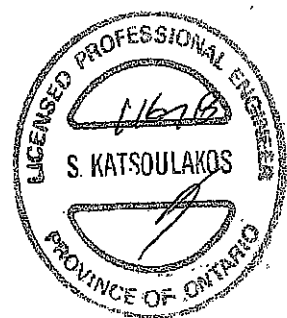
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PU) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 622 2284 1656

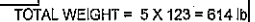
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

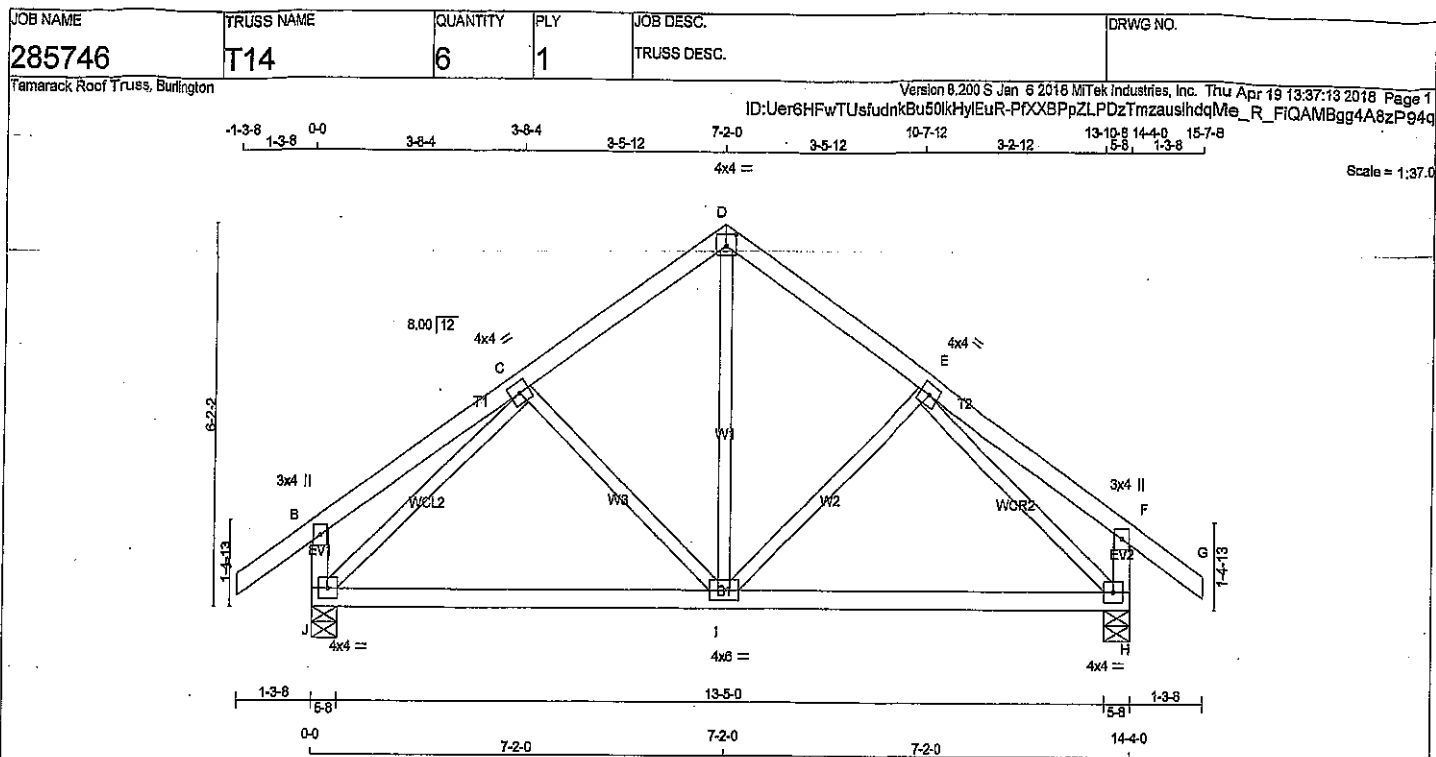
JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL= 0.36 (I) (INPUT = 1.00)



DRWG NO. TAM 3991-13
STRUCTURAL
COMPONENT ONLY



DWG NO. TAM 3993-18
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
J - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMVW1-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	DOWN	HORIZ	UPLIFT
J	920	0	0
H	920	0	0

UNFACTORED REACTIONS

1ST LOASE	MAX/MIN	COMPONENT REACTIONS
JT COMBINED	SNOW	LIVE
J	739	437 / 0
H	739	437 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	CSF (LC)
FR-TO				FR-TO			
A-B	0 / 32	-84.3	0.11 (1)	I-D	0 / 460	0.10 (1)	
B-C	0 / 19	-84.3	0.17 (1)	I-E	-152 / 44	0.06 (1)	
C-D	-645 / 0	-84.3	0.13 (1)	C-I	-152 / 44	0.06 (1)	
D-E	-845 / 0	-84.3	0.13 (1)	J-C	-579 / 0	0.35 (1)	
E-F	0 / 19	-84.3	0.17 (1)	E-H	-579 / 0	0.35 (1)	
F-G	0 / 32	-84.3	0.11 (1)				
J-B	-232 / 0	0.0	0.02 (1)				
H-F	-232 / 0	0.0	0.02 (1)				
J-I	0 / 631	-28.0	0.49 (2)				
I-H	0 / 631	-28.0	0.49 (2)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT. CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.48")
CALCULATED VERT. DEFL.(LL) = L/998 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (0.48")
CALCULATED VERT. DEFL.(TL) = L/998 (0.13")

CSF: TC=0.17/1.00 (E-F:1), BC=0.49/1.00 (H-I:2), WB=0.35/1.00 (E-H:1), SSI=0.17/1.00 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR	SECTION
(PSI)	(PLI)
MAX MIN	MAX MIN
MT20	618 354 1667 822 2284 1658

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.77 (D) (INPUT = 0.90)
JSI METAL= 0.31 (C) (INPUT = 1.00)

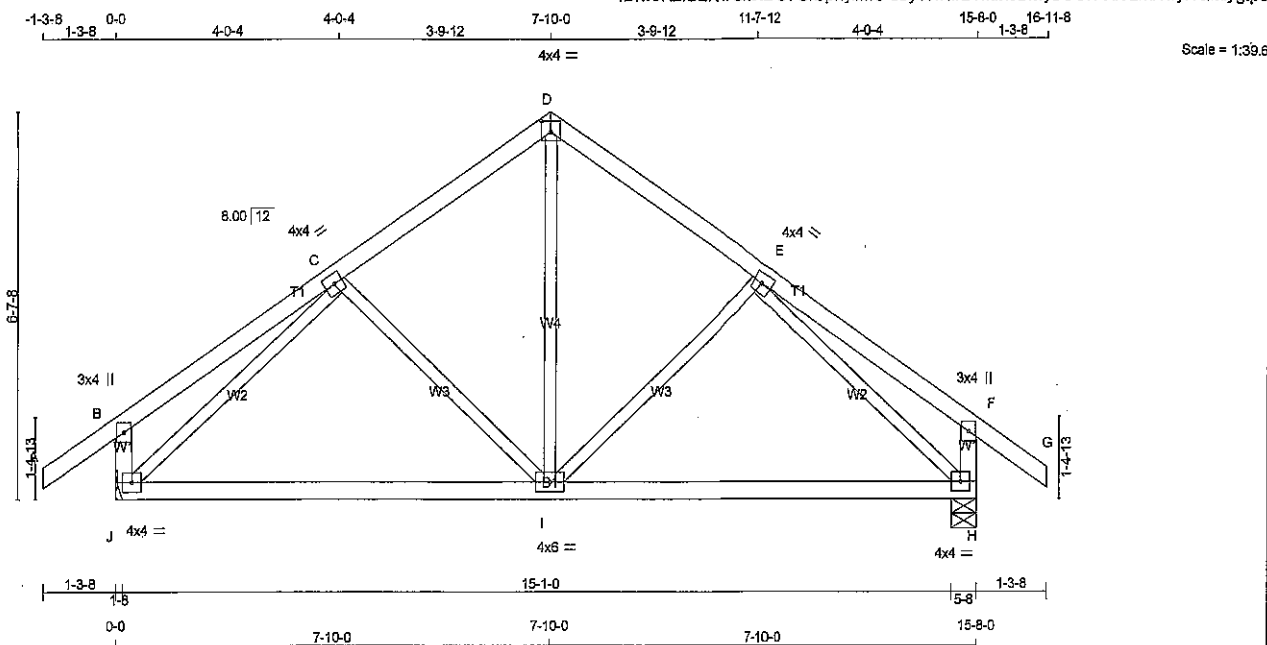


DWG NO. TAM 21218-13
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
285745	T15	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:w3K2IB2K4fGk9tDO7SkJphyfMC-sbyYNHfrBvrkx0BkvjOUCf3otl3zrkHwyNQmlygipe



TOTAL WEIGHT = 67 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
J - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY, SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	4.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMVW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW-t	MT20	4.0	4.0		
I	BMVW-t	MT20	4.0	6.0		
J	BMVW-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD IN-SX
JT	VERT	DOWN	UP	IN-SX
J	995	0	0	5-8
H	995	0	0	5-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
J	800	471 / 0	165 / 0	0 / 0	0 / 0	185 / 0	0 / 0
H	800	471 / 0	165 / 0	0 / 0	0 / 0	185 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 32	-84.3	-84.3 0.11 (1)	I-D	0 / 516	0.12 (1)	
B-C	0 / 21	-84.3	-84.3 0.20 (1)	I-E	-179 / 43	0.09 (1)	
C-D	-717 / 0	-84.3	-84.3 0.16 (1)	C-I	-179 / 43	0.09 (1)	
D-E	-717 / 0	-84.3	-84.3 0.16 (1)	J-C	-975 / 0	0.48 (1)	
E-F	0 / 21	-84.3	-84.3 0.20 (1)	E-H	-975 / 0	0.46 (1)	
F-G	0 / 32	-84.3	-84.3 0.11 (1)				
J-B	-243 / 0	0.0	0.0 0.02 (1)				
H-F	-243 / 0	0.0	0.0 0.02 (1)				
J-I	0 / 709	-28.0	-28.0 0.58 (2)				
I-H	0 / 709	-28.0	-28.0 0.58 (2)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.52")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL) = L/360 (0.52")
CALCULATED VERT. DEFL.(TL) = L/999 (0.18")

CSI: TC=0.20 (B-C:1), BC=0.58 (H-I:2), WB=0.46 (C-I:1), SS=0.19 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (D) (INPUT = 0.90)
JSI METAL= 0.35 (C) (INPUT = 1.00)



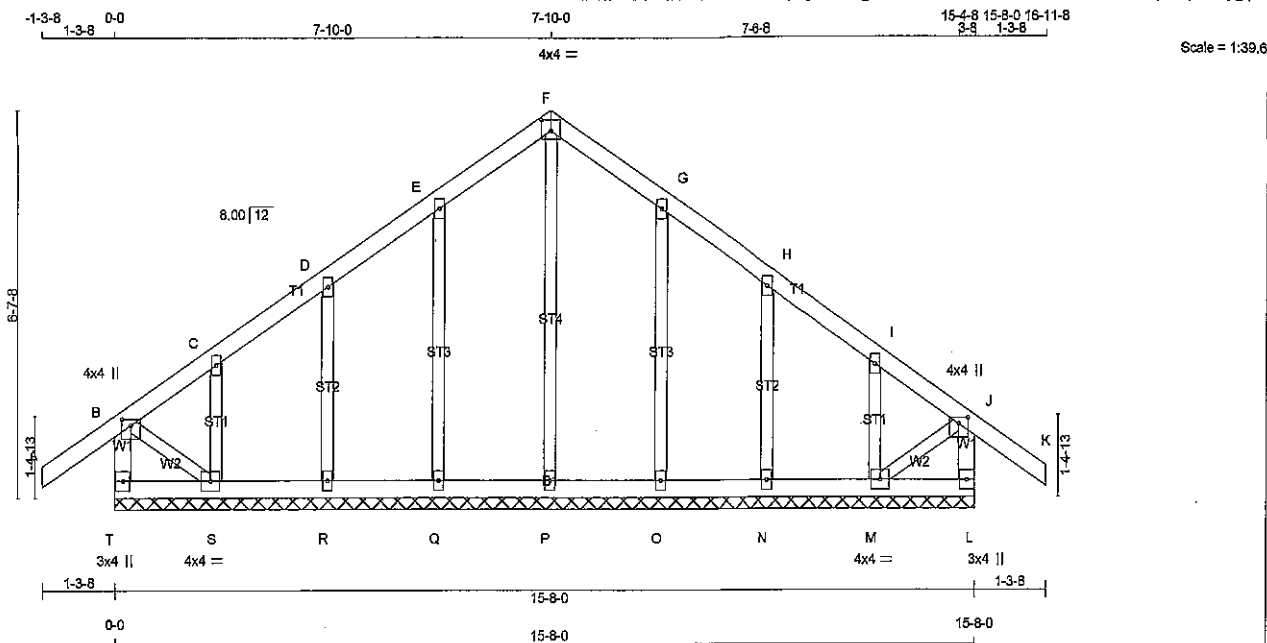
DRWG NO. TAM 4491017
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
285745	G15	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:w3K2IB2K4fGk9ID07SkJphylfMC-8gLMHtYZYra9ePF68oY245ENkr1LdQzpdOyu0KygiPo



TOTAL WEIGHT = 70 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
T - B	2x4	DRY	No.2	SPF
A - F	2x4	DRY	No.2	SPF
F - K	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

GABLE STUDS SPACED AT 2'-0" OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C, D, E, G, H, I						
C	TMVW+w	MT20	2.0	4.0		
F	TTW+p	MT20	4.0	4.0	2.25	2.00
J	TMVW+p	MT20	4.0	4.0	1.25	2.00
L	BMV1+p	MT20	3.0	4.0		
M	BMVW1-t	MT20	4.0	4.0		
N, O, P, Q, R						
N	BMV1+w	MT20	2.0	4.0		
S	BMVW1-t	MT20	4.0	4.0		
T	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
T-B	-253 / 0	0.0	0.0 0.03 (1)	P-F	-129 / 0	0.08 (1)	
A-B	0 / 32	-84.3	-84.3 0.11 (1)	Q-E	-189 / 0	0.08 (1)	
B-C	-48 / 0	-84.3	-84.3 0.11 (1)	R-D	-172 / 0	0.04 (1)	
C-D	-15 / 0	-84.3	-84.3 0.04 (1)	S-C	-108 / 0	0.02 (1)	
D-E	-14 / 0	-84.3	-84.3 0.05 (1)	O-G	-189 / 0	0.08 (1)	
E-F	-21 / 0	-84.3	-84.3 0.05 (1)	N-H	-172 / 0	0.04 (1)	
F-G	-21 / 0	-84.3	-84.3 0.05 (1)	M-I	-108 / 0	0.02 (1)	
G-H	-14 / 0	-84.3	-84.3 0.05 (1)	B-S	0 / 23	0.01 (1)	
H-I	-15 / 0	-84.3	-84.3 0.04 (1)	M-J	0 / 23	0.01 (1)	
I-J	-48 / 0	-84.3	-84.3 0.11 (1)				
J-K	0 / 32	-84.3	-84.3 0.11 (1)				
L-J	-253 / 0	0.0	0.0 0.03 (1)				
T-S	0 / 0	-28.0	-28.0 0.03 (2)				
S-R	0 / 16	-28.0	-28.0 0.03 (2)				
R-Q	0 / 12	-28.0	-28.0 0.02 (2)				
Q-P	0 / 9	-28.0	-28.0 0.02 (2)				
P-O	0 / 9	-28.0	-28.0 0.02 (2)				
O-N	0 / 12	-28.0	-28.0 0.02 (2)				
N-M	0 / 16	-28.0	-28.0 0.03 (2)				
M-L	0 / 0	-28.0	-28.0 0.03 (2)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 089-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11 (J-K:1), BC=0.03 (R-S:2), WB=0.09 (F-P:1), SS=0.08 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 622 2284 1658

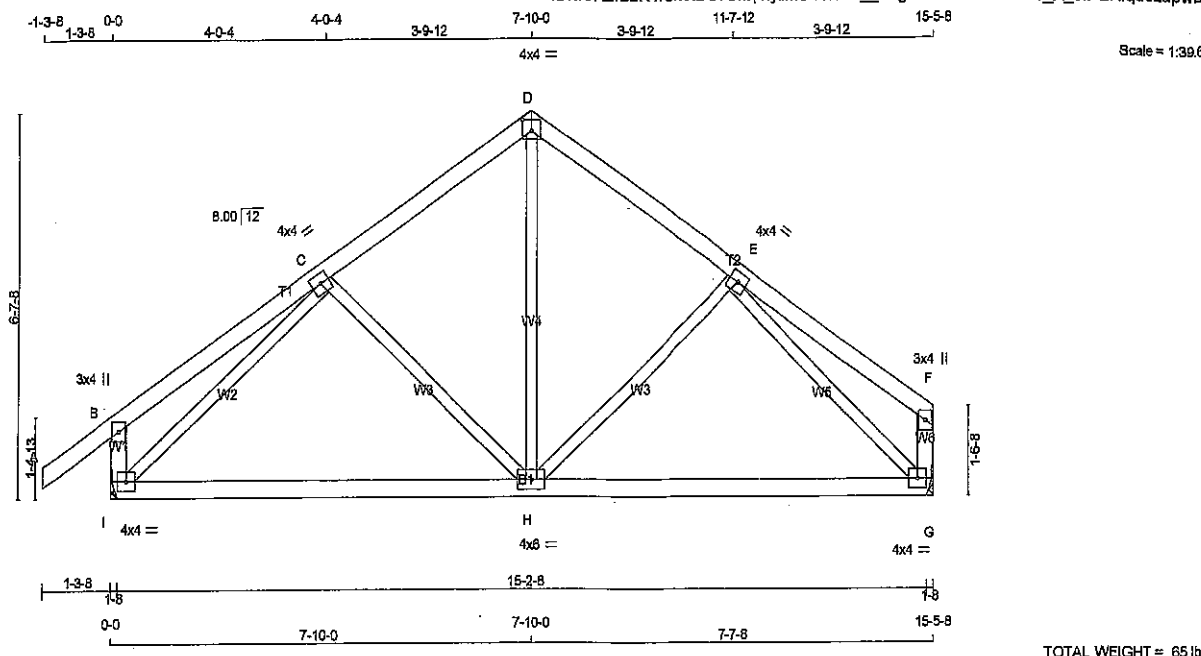
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (B) (INPUT = 0.90)
JSI METAL= 0.05 (G) (INPUT = 1.00)



DRWG NO. TAM 44942-17
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
I	983	0	983	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-3
G	888	0	888	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-3

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	WIND			
I	781	468 / 0	162 / 0	0 / 0	0 / 0	163 / 0	0 / 0
G	712	395 / 0	162 / 0	0 / 0	0 / 0	155 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED		FACTORED		WEBS MAX. FACTORED	
	FORCE (LBS)	VERT. LOAD (PLF)	FROM	TO	FORCE (LBS)	MAX. FACTORED (LC)
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	10.00
B-C	0 / 21	-84.3	-84.3	0.20 (1)	10.00	10.00
C-D	-698 / 0	-84.3	-84.3	0.16 (1)	6.25	6.25
D-E	-687 / 0	-84.3	-84.3	0.15 (1)	6.25	6.25
E-F	0 / 21	-84.3	-84.3	0.19 (1)	10.00	10.00
I-B	-242 / 0	0.0	0.0	0.02 (1)	7.81	7.81
G-F	-118 / 0	0.0	0.0	0.01 (1)	7.81	7.81
I-H	0 / 696	-28.0	-28.0	0.56 (2)	10.00	10.00
H-G	0 / 672	-28.0	-28.0	0.56 (2)	10.00	10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.52")

CALCULATED VERT. DEFL.(LL)= L/999 (0.11")

ALLOWABLE DEFL.(TL)= L/360 (0.52")

CALCULATED VERT. DEFL.(TL)= L/981 (0.19")

CS: TC=0.20/1.00 (B-C:1), BC=0.58/1.00 (H-I:2), WB=0.45/1.00 (C-I:1), SSI=0.19/1.00 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

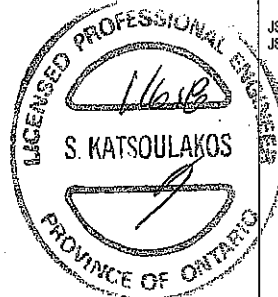
PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (D) (INPUT = 0.90)

JSI METAL= 0.34 (C) (INPUT = 1.00)



DRWG NO. TAM 4010 -13

STRUCTURAL

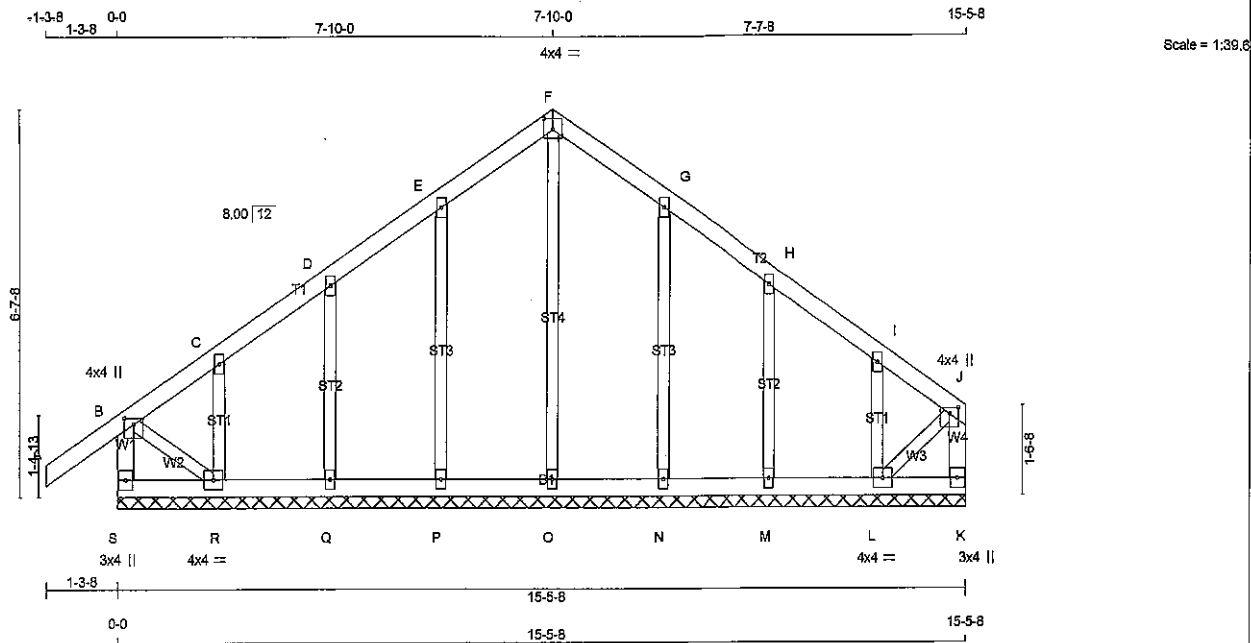
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
285745	G15A	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:w3K2B2K4fGk9ID07SkJphylfMC-csv8UCZBJ8i0GZqSIW3HdJnYUFNaMthHys2hRYmygipn



TOTAL WEIGHT = 68 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
S - B	2x4	DRY	No.2	SPF
A - F	2x4	DRY	No.2	SPF
F - J	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
S - K	2x4	DRY	No.2	SPF

ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

GABLE STUDS SPACED AT 2'-0" OC.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	4.0	1.25	2.00
C, D, E, G, H, I					
C TMW+w	MT20	2.0	4.0		
F TTW-p	MT20	4.0	4.0	2.25	2.00
J TMVW+p	MT20	4.0	4.0	1.25	2.00
K BMV1+p	MT20	3.0	4.0		
L BMWW1-l	MT20	4.0	4.0		
M, N, O, P, Q					
M BMW1+w	MT20	2.0	4.0		
R BMWW1-l	MT20	4.0	4.0		
S BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
S-B	-258 / 0	0.0 0.0 0.03 (1)	7.81	O-F	-124 / 0	0.09 (1)	
A-B	0 / 32	-84.3 -84.3 0.11 (1)	10.00	P-E	-189 / 0	0.08 (1)	
B-C	-50 / 0	-84.3 -84.3 0.11 (1)	8.25	Q-D	-172 / 0	0.04 (1)	
C-D	-19 / 0	-84.3 -84.3 0.04 (1)	6.25	R-C	-108 / 0	0.02 (1)	
D-E	-18 / 0	-84.3 -84.3 0.05 (1)	6.25	N-G	-191 / 0	0.08 (1)	
E-F	-25 / 0	-84.3 -84.3 0.05 (1)	6.25	M-H	-162 / 0	0.04 (1)	
F-G	-25 / 0	-84.3 -84.3 0.05 (1)	6.25	L-I	-162 / 0	0.03 (1)	
G-H	-17 / 0	-84.3 -84.3 0.05 (1)	6.25	B-R	0 / 28	0.01 (1)	
H-I	-24 / 0	-84.3 -84.3 0.04 (1)	6.25	L-J	0 / 32	0.01 (1)	
I-J	-21 / 0	-84.3 -84.3 0.04 (1)	6.25				
K-J	-84 / 0	0.0 0.0 0.01 (1)	7.81				
S-R	0 / 0	-28.0 -28.0 0.03 (2)	10.00				
R-Q	0 / 20	-28.0 -28.0 0.03 (2)	10.00				
Q-P	0 / 16	-28.0 -28.0 0.02 (2)	10.00				
P-O	0 / 13	-28.0 -28.0 0.02 (2)	10.00				
O-N	0 / 13	-28.0 -28.0 0.02 (2)	10.00				
N-M	0 / 16	-28.0 -28.0 0.02 (2)	10.00				
M-L	0 / 20	-28.0 -28.0 0.02 (2)	10.00				
L-K	0 / 0	-28.0 -28.0 0.02 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 3.0 PSF
BOT CH. LL	= 10.5 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11 (A-B:1), BC=0.03 (Q-R:2), WB=0.09 (F-O:1), SS=0.08 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	618 354	1667 822	2284 1656

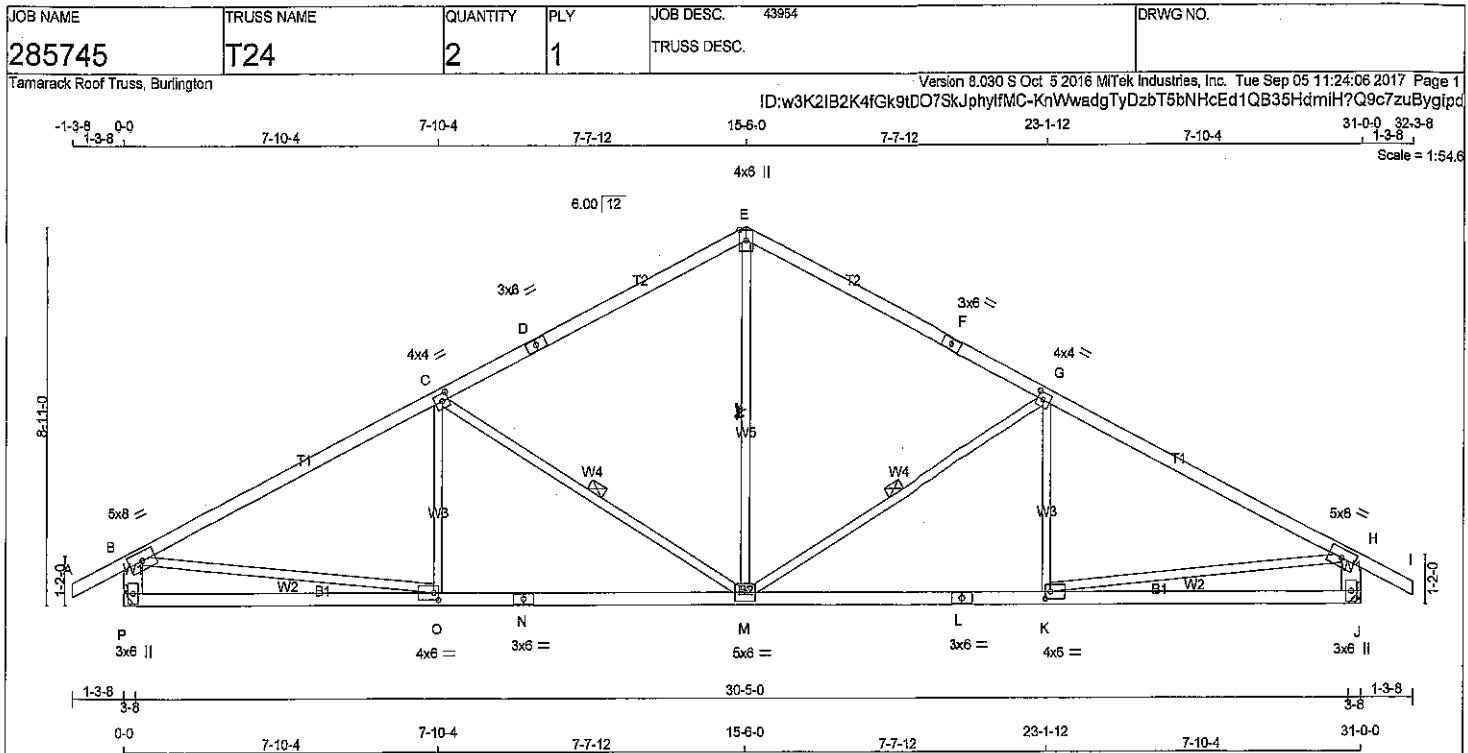
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (B) (INPUT = 0.90)
JSI METAL= 0.05 (G) (INPUT = 1.00)



DWG NO. TAM 4494317
STRUCTURAL
COMPONENT ONLY



LUMBER
N. L. G. A. RULES
CHORDS SIZE LUMBER DESCR.

A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
P - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY, SEASONED LUMBER.

PLATES (table is in inches)

JT. TYPE	PLATES	W	LEN	Y	X
B TMVW-t	MT20	5.0	8.0		
C TMVW-t	MT20	4.0	4.0	2.00	1.75
D TS-t	MT20	3.0	8.0		
E TTW+p	MT20	4.0	6.0	Edge	
F TS-t	MT20	3.0	6.0		
G TMVW-t	MT20	4.0	4.0	2.00	1.75
H TMVW-t	MT20	5.0	8.0		
J BMV1+p	MT20	3.0	6.0		
K BMVW-t	MT20	4.0	8.0	2.00	1.50
L BS-t	MT20	3.0	6.0		
M BMVW-t	MT20	5.0	8.0		
N BS-t	MT20	3.0	6.0		
O BMVW-t	MT20	4.0	6.0	2.00	1.50
P BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	FACTORED GROSS REACTION HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	MAXIMUM FACTORED GROSS REACTION HORZ	INPUT BRG	REQD BRG
P	1854	0	1854	0	0	HANGER BY OTHERS
J	1854	0	1854	0	0	MIN. SEAT SIZE: 3-8 HANGER BY OTHERS MIN. SEAT SIZE: 3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX. MIN. SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
P	1508	863 / 0	326 / 0	0 / 0	0 / 0	318 / 0	0 / 0
J	1508	863 / 0	326 / 0	0 / 0	0 / 0	318 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.27 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-M, C-M. *ENV*

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LOAD LC1	MAX CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM	TO			FR-TO			
A-B	0 / 26	-84.3	-84.3	0.11 (1)	10.00	M-E	0 / 1098	0.25 (1)	
B-C	-2529 / 0	-84.3	-84.3	0.94 (1)	3.27	M-G	-840 / 0	0.48 (1)	
C-D	-1803 / 0	-84.3	-84.3	0.78 (1)	3.96	K-G	-9 / 307	0.07 (3)	
D-E	-1803 / 0	-84.3	-84.3	0.78 (1)	3.96	C-M	-840 / 0	0.48 (1)	
E-F	-1803 / 0	-84.3	-84.3	0.78 (1)	3.96	O-C	-9 / 307	0.07 (3)	
F-G	-1803 / 0	-84.3	-84.3	0.78 (1)	3.96	B-O	0 / 2306	0.52 (1)	
G-H	-2529 / 0	-84.3	-84.3	0.94 (1)	3.27	K-H	0 / 2306	0.52 (1)	
H-I	0 / 26	-84.3	-84.3	0.11 (1)	10.00				
P-B	-1764 / 0	0.0	0.0	0.11 (1)	7.48				
J-H	-1764 / 0	0.0	0.0	0.11 (1)	7.48				
P-O	0 / 0	-28.0	-28.0	0.42 (3)	10.00				
O-N	0 / 2293	-28.0	-28.0	0.68 (2)	10.00				
N-M	0 / 2293	-28.0	-28.0	0.68 (2)	10.00				
M-L	0 / 2293	-28.0	-28.0	0.68 (2)	10.00				
L-K	0 / 2293	-28.0	-28.0	0.68 (2)	10.00				
K-J	0 / 0	-28.0	-28.0	0.42 (3)	10.00				

ED PROFESSIONAL

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/999 (0.18")
ALLOWABLE DEFL.(TL) = L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/999 (0.30")

CSI: TC=0.94 (G-H:1), BC=0.68 (K-M:2),
WB=0.52 (H-K:1), SS=0.28 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

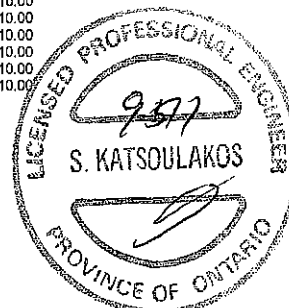
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (K) (INPUT = 0.90)
JSI METAL= 0.60 (N) (INPUT = 1.00)



DRWG NO. TAM 44912-17
STRUCTURAL
COMPONENT ONLY



DRY: SEASONED LUMBER

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

1) SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 244.5 lbs FACTORED DOWN AT 4-0-11,
101.4 lbs FACTORED DOWN AT 5-3-12, 101.4 lbs
FACTORED DOWN AT 7-3-12, 101.4 lbs
FACTORED DOWN AT 9-3-12, 101.4 lbs
FACTORED DOWN AT 11-3-12, AND 101.4 lbs
FACTORED DOWN AT 13-3-12, AND 123.4 lbs
FACTORED DOWN AT 15-3-12 ON TOP CHORD,
AND 69.9 lbs FACTORED DOWN AT 1-3-12, 69.9
lbs FACTORED DOWN AT 3-3-12, 69.9 lbs
FACTORED DOWN AT 5-3-12, 69.9 lbs
FACTORED DOWN AT 7-3-12, 69.9 lbs
FACTORED DOWN AT 9-3-12, 69.9 lbs
FACTORED DOWN AT 11-3-12, AND 69.9 lbs
FACTORED DOWN AT 13-3-12, AND 82.7 lbs
FACTORED DOWN AT 15-3-12 ON BOTTOM
CHORD. DESIGN FOR UNSPECIFIED
CONNECTION(S) IS DELEGATED TO THE
BUILDING DESIGNER.

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE
C	4-0-11	-23	-25	--	FRONT	VERT	DEAD
C	4-0-11	-222	-222	--	FRONT	VERT	SNOW
L	5-3-12	-101	-101	--	BACK	VERT	TOTAL
M	7-3-12	-101	-101	--	BACK	VERT	TOTAL
N	9-3-12	-101	-101	--	BACK	VERT	TOTAL
O	11-3-12	-101	-101	--	BACK	VERT	TOTAL
P	13-3-12	-101	-101	--	BACK	VERT	TOTAL
Q	15-3-12	-123	-123	--	BACK	VERT	TOTAL
R	1-3-12	-40	-70	--	BACK	VERT	TOTAL
S	3-3-12	-40	-70	--	BACK	VERT	TOTAL

PLATE ROTATION TOL = 5.0 Deg.



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286043	T30	1	1	TRUSS DESC.	

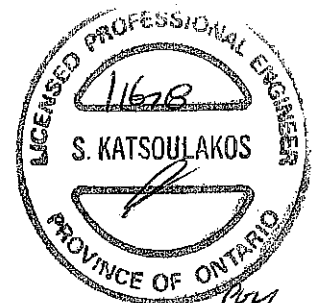
Tamarack Roof Truss, Burlington

Version 8.200 S Dec 12 2017 MiTek Industries, Inc. Tue Jan 16 11:20:07 2018 Page 2

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FACTORED CONCENTRATED LOADS (LBS)

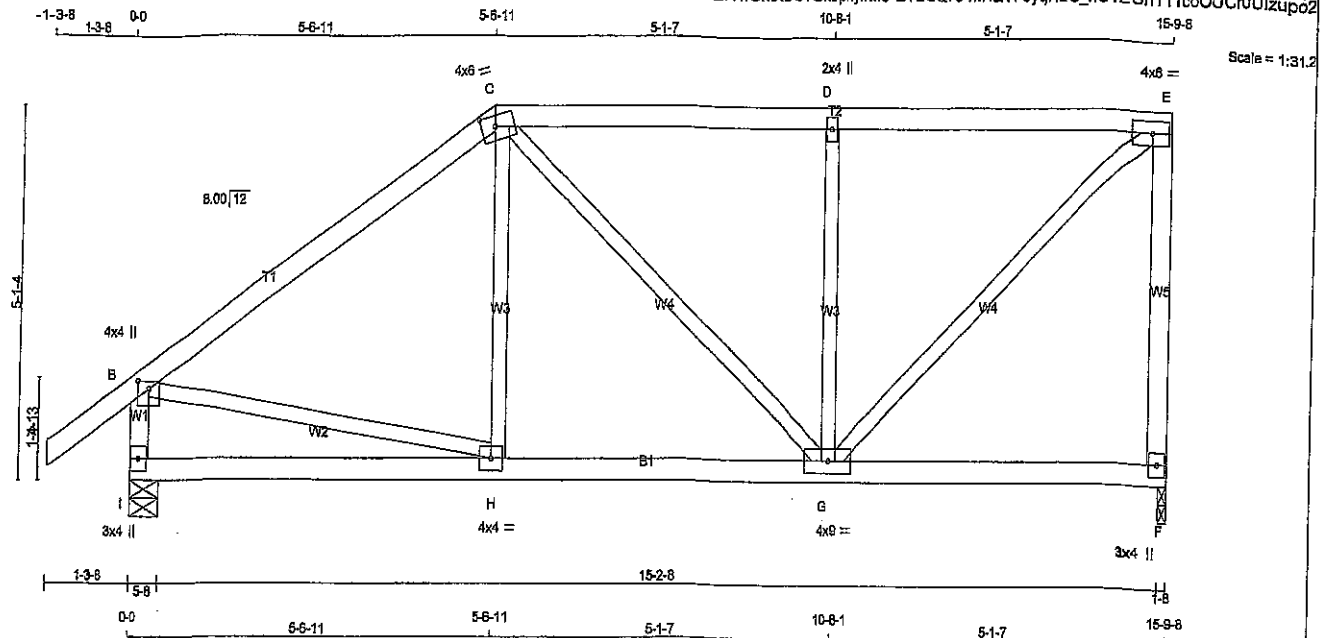
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
T	5-3-12	-40	-70	—	BACK	VERT	TOTAL
U	7-3-12	-40	-70	—	BACK	VERT	TOTAL
V	9-3-12	-40	-70	—	BACK	VERT	TOTAL
W	11-3-12	-40	-70	—	BACK	VERT	TOTAL
X	13-3-12	-40	-70	—	BACK	VERT	TOTAL
Y	15-3-12	-47	-83	—	BACK	VERT	TOTAL



DWG NO. TAM 3995 -18
STRUCTURAL
COMPONENT ONLY

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 68 lb

LUMBER	N. L. G. A. RULES	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMW+p	MT20	4.0	4.0	1.25	2.00
C	TTWW-m	MT20	4.0	8.0	1.75	2.50
D	TMW+w	MT20	2.0	4.0		
E	TMW+I	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWW-t	MT20	4.0	9.0		
H	BMWW-I	MT20	4.0	4.0		
I	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
F	886	0	886	0	1-8	1-8
I	1002	0	1002	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	WIND			
F	726	404 / 0	166 / 0	0 / 0	0 / 0	158 / 0	0 / 0
I	806	474 / 0	166 / 0	0 / 0	0 / 0	166 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX FACTORED CSI (LC)	
FR-TO		FROM TO			FR-TO			
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	H-C	0 / 222	0.05 (3)
B-C	-821 / 0	-84.3	-84.3	0.49 (1)	5.96	C-G	-23 / 33	0.02 (3)
C-D	-707 / 0	-84.3	-84.3	0.38 (1)	8.25	G-D	-532 / 0	0.20 (1)
D-E	-707 / 0	-84.3	-84.3	0.38 (1)	6.25	G-E	0 / 957	0.22 (1)
F-E	-826 / 0	0.0	0.0	0.36 (1)	7.81	B-H	0 / 695	0.16 (1)
I-B	-937 / 0	0.0	0.0	0.10 (1)	7.81			
I-H	0 / 0	-26.0	-26.0	0.19 (3)	10.00			
H-G	0 / 683	-26.0	-26.0	0.26 (2)	10.00			
G-F	0 / 0	-26.0	-26.0	0.16 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6	PSF
DL	= 3.0	PSF
BOT CH. LL	= 10.5	PSF
DL	= 7.0	PSF
TOTAL LOAD	= 46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.09/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.53")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.05")
ALLOWABLE DEFL. (TL) = $L/360$ (0.53")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.07")

CSI: TC=0.49/1.00 (B-C:1), BC=0.28/1.00 (G-H:2), WB=0.22/1.00 (E-G:1), SS=0.21/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	618	354	1887
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (B) (INPUT = 0.80)

JSI METAL= 0.28 (H) (INPUT = 1.00)

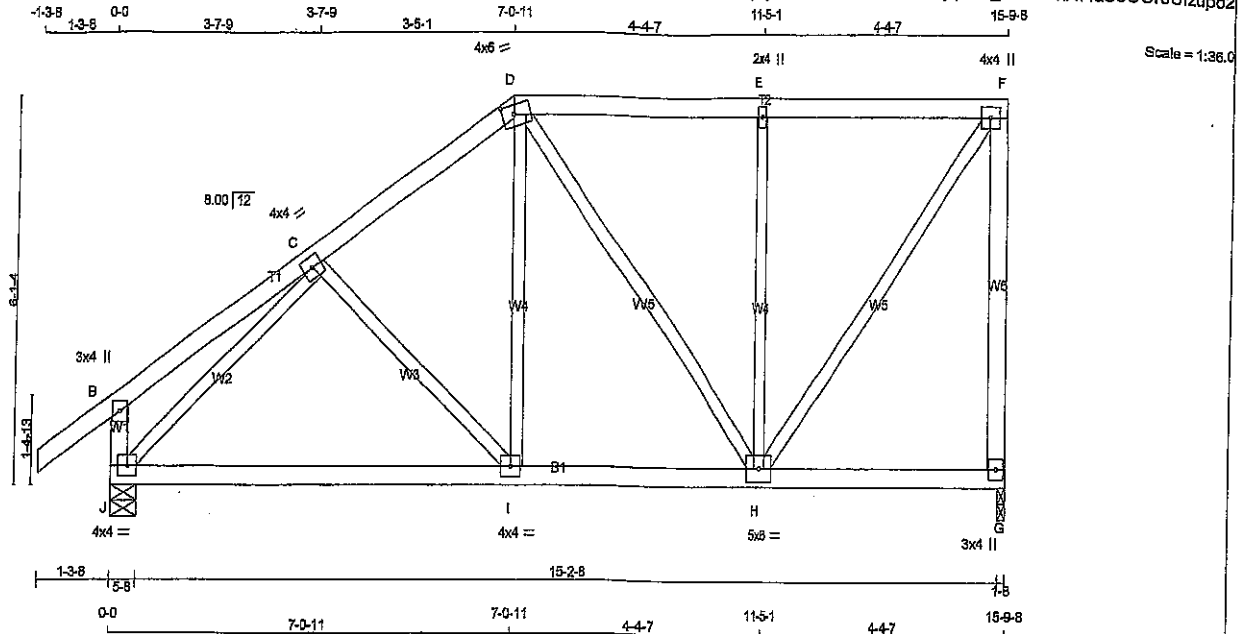


DRWG NO. TAM 3996-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286043	T32	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 75 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESC.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
J - B	2x4	DRY	No.2	SPF	
J - G	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0			
C	TMVW-t	MT20	4.0	4.0			
D	TTWW-m	MT20	4.0	6.0	1.75	2.50	
E	TMVW+p	MT20	2.0	4.0			
F	TMVW+p	MT20	4.0	4.0			
G	BMV1+p	MT20	3.0	4.0			
H	BMVWV-t	MT20	5.0	6.0			
I	BMVW-t	MT20	4.0	4.0			
J	BMVW-t	MT20	4.0	4.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
G	886	0	886	0	0	1-8	1-8		
J	1002	0	1002	0	0	5-8	5-8		

UNFACTORED REACTIONS						
JT	1ST CASE	MAX	MIN	COMPONENT	REACTIONS	
G	728	404/0	166/0	PERM	LIVE	DEAD
J	806	474/0	166/0	PERM	LIVE	DEAD

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING				TOTAL LOAD CASES: (4)			
CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	
FR-TO		FROM TO		FR-TO			
A-B	0/32	-84.3 -84.3	0.11 (1)	10.00	C-I	-127/52	0.05 (1)
B-C	0/18	-84.3 -84.3	0.16 (1)	10.00	D-H	-166/0	0.08 (2)
C-D	-759/0	-84.3 -84.3	0.17 (1)	6.25	H-E	-453/0	0.17 (2)
D-E	-522/0	-84.3 -84.3	0.27 (1)	6.25	E-F	0/684	0.27 (1)
E-F	-522/0	-84.3 -84.3	0.27 (1)	6.25	F-G	-987/0	0.19 (1)
G-F	-931/0	0.0 0.0	0.60 (1)	7.61	J-C		0.38 (1)
J-B	-231/0	0.0 0.0	0.02 (1)	7.61			
J-I	0/707	-28.0 -28.0	0.38 (2)	10.00			
I-H	0/619	-28.0 -28.0	0.38 (2)	10.00			
H-G	0/0	-28.0 -28.0	0.13 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAO = 45.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.53")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL) = L/360 (0.53")
CALCULATED VERT. DEFL.(TL) = L/999 (0.18")

CSI: TO=0.60/1.00 (F-G:1), BC=0.38/1.00 (H-I:2), WB=0.38/1.00 (C-J:1), SSI=0.18/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

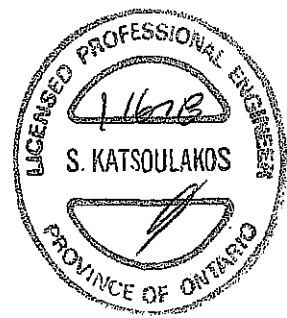
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1967 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JBI GRIP=0.88 (F) (INPUT = 0.80)
JBI METAL=0.35 (C) (INPUT = 1.00)

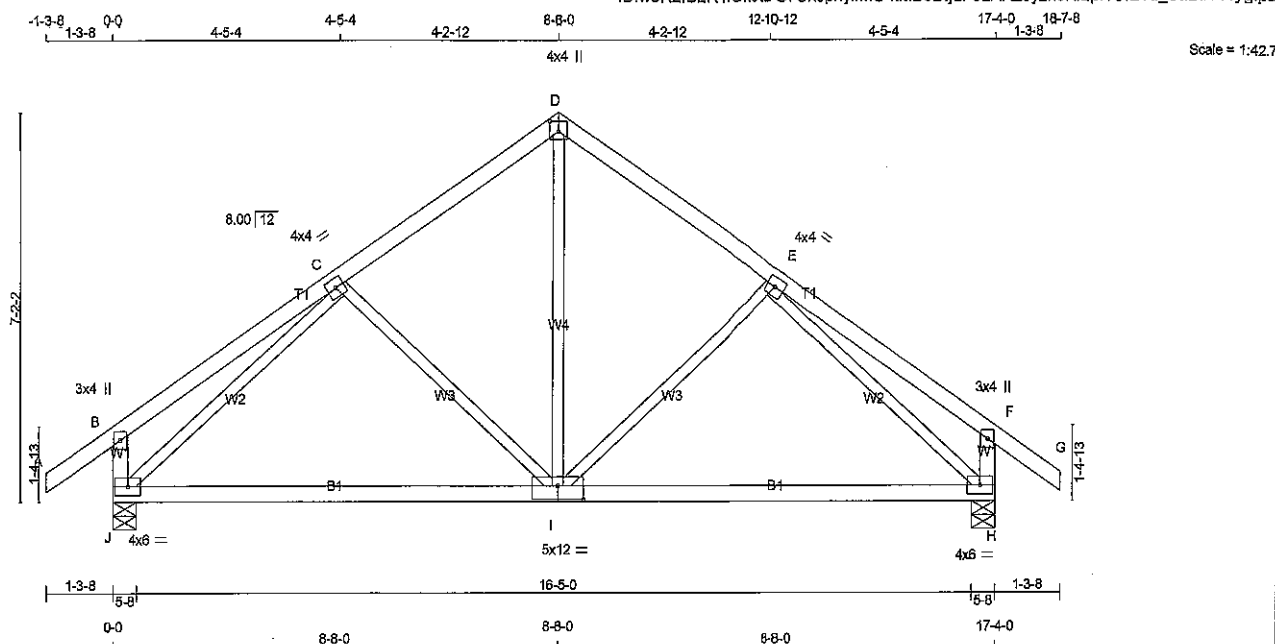


DRWG NO. TAM 3997-18
STRUCTURAL
COMPONENT ONLY

JOB NAME 285745	TRUSS NAME T34	QUANTITY 1	PLY 1	JOB DESC. 43954 TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:w3K2IB2K4fGk9ID07SkJphyfMC-kMB3DfjLF8LAKZJyzkoKf2p1Uf2vd_traLdVWygipa



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
J - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
J - I	2x4	DRY	No.2
I - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	3.0	4.0
C	TMWW-t	MT20	4.0	4.0
D	TTW+p	MT20	4.0	4.0 2.25 2.00
E	TMWW-t	MT20	4.0	4.0
F	TMV+p	MT20	3.0	4.0
H	BMVW1-t	MT20	4.0	6.0
I	BSWWH-t	MT20	5.0	12.0 3.00 6.00
J	BMVW1-t	MT20	4.0	6.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
J	1089	0	1089	0	5-8
H	1089	0	1089	0	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. SNOW	MAX./MIN. LIVE	MAX./MIN. WIND	MAX./MIN. DEAD	MAX./MIN. SOIL
J	877	514 / 0	182 / 0	0 / 0	182 / 0	0 / 0
H	877	514 / 0	182 / 0	0 / 0	182 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX. FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	LC1 MAX	UNBRAC LENGTH	MEMB.	FORCE (LBS)	LC1 MAX	LC1 MAX
FR-TO		FROM	TO	CSI (LC)		FR-TO		FROM	TO
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	I-D	0 / 587	0.13 (1)	
B-C	0 / 23	-84.3	-84.3	0.25 (1)	10.00	I-E	-212 / 42	0.13 (1)	
C-D	-807 / 0	-84.3	-84.3	0.20 (1)	6.25	C-I	-212 / 42	0.13 (1)	
D-E	-807 / 0	-84.3	-84.3	0.20 (1)	6.25	J-C	-1097 / 0	0.63 (1)	
E-F	0 / 23	-84.3	-84.3	0.25 (1)	10.00	E-H	-1097 / 0	0.63 (1)	
F-G	0 / 32	-84.3	-84.3	0.11 (1)	10.00				
J-B	-255 / 0	0.0	0.0	0.03 (1)	7.81				
H-F	-255 / 0	0.0	0.0	0.03 (1)	7.81				
J-I	0 / 809	-28.0	-28.0	0.70 (2)	10.00				
I-H	0 / 809	-28.0	-28.0	0.70 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.58")
CALCULATED VERT. DEFL.(LL)= L/998 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.58")
CALCULATED VERT. DEFL.(TL)= L/766 (0.27")

CSI: TC=0.25 (E-F:1), BC=0.70 (H-I:2), WB=0.63 (E-H:1), SI=0.21 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (E) (INPUT = 0.90)
JSI METAL= 0.59 (I) (INPUT = 1.00)



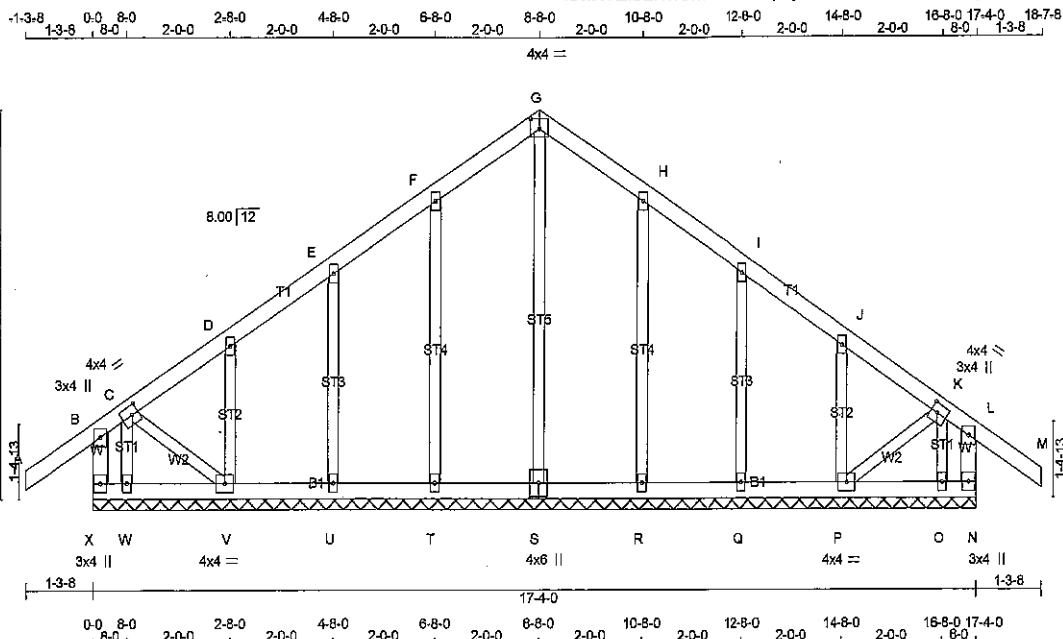
DWG NO. TAM 44917-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
285745	G34	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:w3K2IB2K4fGk9ID07SkJphylfMC-csv8UCZBJ8IGZqSiW3HdJnYUFNBmI2ys2hRYmygipn



Scale = 1/4" = 1'-0"

TOTAL WEIGHT = 80 lb

LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES				
A - G	2x4	DRY	No.2	SPF
G - M	2x4	DRY	No.2	SPF
X - B	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
X - S	2x4	DRY	No.2	SPF
S - N	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
ALL GABLE WEBS 2x3 DRY No.2 SPF
DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2'-0" OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-I	MT20	4.0	4.0	2.00	1.50
D, E, F, H, I, J						
D	TMW+w	MT20	2.0	4.0		
G	TTW-p	MT20	4.0	4.0	2.25	2.00
K	TMWW-I	MT20	4.0	4.0	2.00	1.50
L	TMV+p	MT20	3.0	4.0		
N	BMV1+p	MT20	3.0	4.0		
O, Q, R, T, U, W						
O	BMW1+w	MT20	2.0	4.0		
P	BMWW1-I	MT20	4.0	4.0		
S	BSW1-I	MT20	4.0	6.0		
V	BMWW1-I	MT20	4.0	4.0		
X	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD LC1 (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0/32	-84.3 -84.3 0.11 (1)	10.00	S-G	-117/0	0.10 (1)	
B-C	-55/0	-84.3 -84.3 0.11 (1)	6.25	T-F	-192/0	0.10 (1)	
C-D	-30/0	-84.3 -84.3 0.04 (1)	6.25	U-E	-160/0	0.05 (1)	
D-E	-30/0	-84.3 -84.3 0.04 (1)	6.25	V-D	-172/0	0.03 (1)	
E-F	-23/0	-84.3 -84.3 0.05 (1)	6.25	W-C	-42/0	0.01 (1)	
F-G	-31/0	-84.3 -84.3 0.05 (1)	6.25	R-H	-192/0	0.10 (1)	
G-H	-31/0	-84.3 -84.3 0.05 (1)	6.25	Q-I	-160/0	0.05 (1)	
H-I	-23/0	-84.3 -84.3 0.05 (1)	6.25	P-J	-172/0	0.03 (1)	
I-J	-30/0	-84.3 -84.3 0.04 (1)	6.25	O-K	-42/0	0.01 (1)	
J-K	-30/0	-84.3 -84.3 0.04 (1)	6.25	C-V	0/37	0.01 (1)	
K-L	-55/0	-84.3 -84.3 0.11 (1)	6.25	P-K	0/37	0.01 (1)	
L-M	0/32	-84.3 -84.3 0.11 (1)	10.00				
X-B	-244/0	0.0 0.0 0.03 (1)	7.81				
N-L	-244/0	0.0 0.0 0.03 (1)	7.81				
X-W	0/0	-28.0 -28.0 0.02 (2)	10.00				
W-V	0/0	-28.0 -28.0 0.02 (2)	10.00				
V-U	0/24	-28.0 -28.0 0.03 (2)	10.00				
U-T	0/21	-28.0 -28.0 0.02 (2)	10.00				
T-S	0/18	-28.0 -28.0 0.02 (2)	10.00				
S-R	0/18	-28.0 -28.0 0.02 (2)	10.00				
R-Q	0/21	-28.0 -28.0 0.02 (2)	10.00				
Q-P	0/24	-28.0 -28.0 0.03 (2)	10.00				
P-O	0/0	-28.0 -28.0 0.02 (2)	10.00				
O-N	0/0	-28.0 -28.0 0.02 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPC 2011

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11 (A-B:1), BC=0.03 (P-Q:2), WB=0.10 (G-S:1), SSI=0.08 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

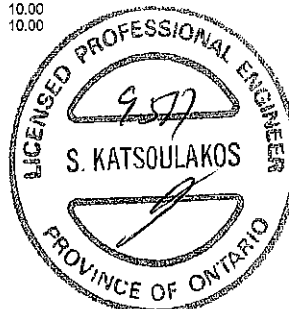
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	818 354	1667 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.51 (C) (INPUT = 0.90)
JSI METAL= 0.05 (S) (INPUT = 1.00)

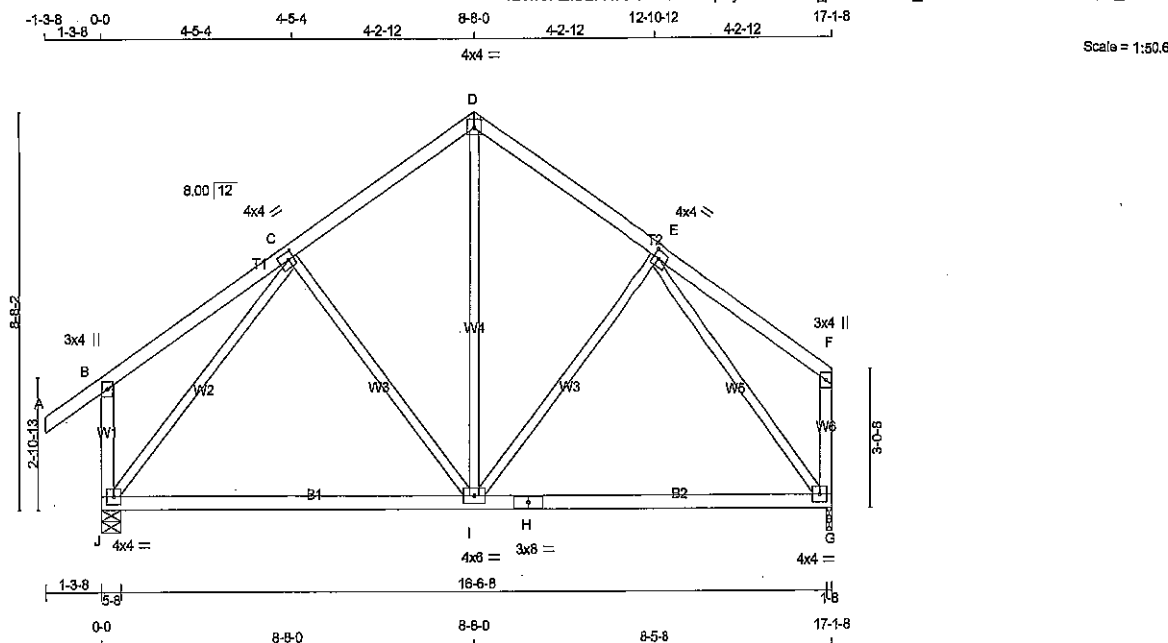


DWG NO. TAM 44944-17
STRUCTURAL
COMPONENT ONLY

Tamarack Roof Truss, Burlington

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Scale = 1:50.6

TOTAL WEIGHT = 79 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
J - B	2x4	DRY	No.2
G - F	2x4	DRY	No.2
J - H	2x4	DRY	No.2
H - G	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0	2.00	1.50
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	DOWN	BRG	IN-SX	BRG	IN-SX
J	1077	0	1077	0	0	5-8	5-8		
G	981	0	981	0	0	1-8	1-8		

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
J	COMBINED	508 / 0	180 / 0	0 / 0	0 / 0	179 / 0	0 / 0
G		436 / 0	180 / 0	0 / 0	0 / 0	171 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	MAX. FACTORED	VERT. LOAD	LC1 MAX	MEMB.	MAX. FACTORED	FORCE	MAX
	(LBS)	(PLF)	CSI (LC)		(LBS)	(LBS)	CSI (LC)
FR-TO		FROM	TO	FR-TO			
A-B	0 / 32	-84.3	-84.3 0.11 (1)	10.00	C-I	-89 / 90	0.08 (1)
B-C	0 / 24	-84.3	-84.3 0.28 (1)	10.00	I-D	0 / 438	0.10 (2)
C-D	-651 / 0	-84.3	-84.3 0.20 (1)	6.25	I-E	-59 / 101	0.05 (1)
D-E	-650 / 0	-84.3	-84.3 0.19 (1)	6.25	J-C	-931 / 0	0.83 (1)
E-F	0 / 24	-84.3	-84.3 0.25 (1)	10.00	E-G	-929 / 0	0.79 (1)
J-B	-254 / 0	0.0	0.0 0.04 (1)	7.81			
G-F	-130 / 0	0.0	0.0 0.02 (1)	7.81			
J-I	0 / 578	-28.0	-28.0 0.87 (2)	10.00			
I-H	0 / 559	-28.0	-28.0 0.86 (2)	10.00			
H-G	0 / 559	-28.0	-28.0 0.86 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.57")
CALCULATED VERT. DEFL. (LL) = L/999 (0.18")
ALLOWABLE DEFL. (TL) = L/360 (0.57")
CALCULATED VERT. DEFL. (TL) = L/751 (0.27")

CSI: TC=0.28/1.00 (B-C:1), BC=0.67/1.00 (I-J:2), WB=0.83/1.00 (C-J:1), SS=0.21/1.00 (J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	618	354	1687
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (E) (INPUT = 0.90)
JSI METAL= 0.46 (H) (INPUT = 1.00)



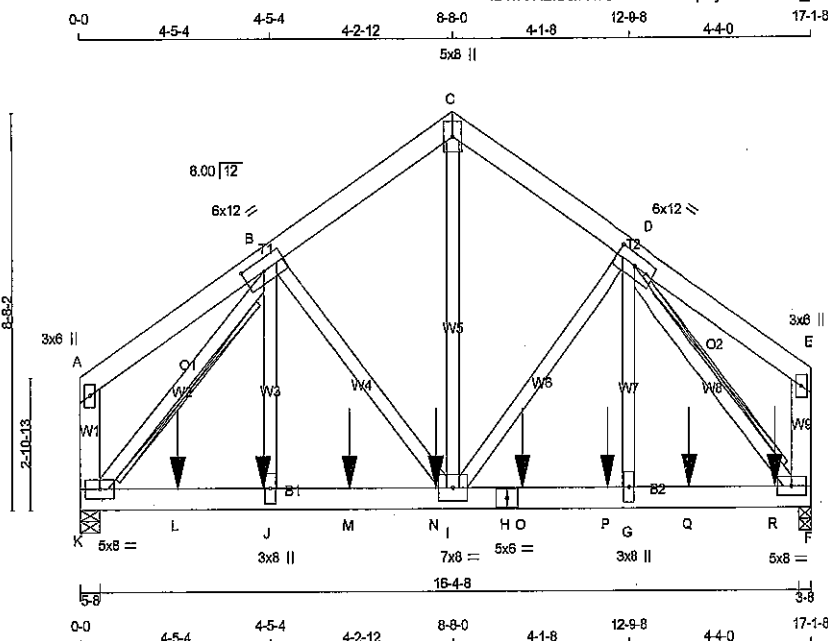
DWG NO. TAW 3339-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285748	T36A	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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Scale = 1:50.6

TOTAL WEIGHT = 3 X 125 = 375 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
K - A	2x6	DRY	No.2	SPF
F - E	2x6	DRY	No.2	SPF
K - H	2x6	DRY	2100F 1.8E	SPF
H - F	2x6	DRY	2100F 1.8E	SPF

ALL WEBS 2x4 DRY No.2 SPF EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
A - C	2	12
C - E	2	12
K - A	2	12
F - E	2	12
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
K - H	2	9
H - F	2	8
WEBS: (0.122"x3") SPIRAL NAILS		
2x4	1	6

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	6.0	
B	TMVWW-t	MT20	6.0	12.0	3.00 5.75
C	TTW+p	MT20	5.0	8.0	
D	TMVWW-t	MT20	6.0	12.0	3.00 5.75
E	TMV+p	MT20	3.0	6.0	
F	BMVW-t	MT20	5.0	8.0	
G	BMV+w	MT20	3.0	8.0	
H	BS-t	MT20	5.0	8.0	
I	BMVWW-t	MT20	7.0	8.0	
J	BMV+w	MT20	3.0	8.0	
K	BMVW-t	MT20	5.0	8.0	

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
K	7383	0	7383	0	0	5-8	5-8
F	8466	0	8466	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. SNOW	MIN. LIVE	PERM. LIVE	WIND	DEAD	SOIL
K	5995	3436 / 0	1294 / 0	0 / 0	0 / 0	1283 / 0	0 / 0
F	6889	3924 / 0	1503 / 0	0 / 0	0 / 0	1462 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.84 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x6 DRY SPF No.2 T-BRACE AT B-K, D-F

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 8" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MEMB.	FORCE (LBS)
FR-TO		FROM TO	FR-TO	
A-B	-17 / 0	-84.3 -84.3	0.08 (1)	6.25
B-C	-5491 / 0	-84.3 -84.3	0.07 (1)	5.84
C-D	-5489 / 0	-84.3 -84.3	0.07 (1)	5.84
D-E	-18 / 0	-84.3 -84.3	0.06 (1)	6.25
K-A	-206 / 0	0.0 0.0	0.01 (1)	7.81
F-E	-205 / 0	0.0 0.0	0.01 (1)	7.81
K-L	0 / 5340	-28.0 -28.0	0.27 (1)	10.00
L-J	0 / 5340	-28.0 -28.0	0.27 (1)	10.00
J-M	0 / 5392	-28.0 -28.0	0.25 (1)	10.00
M-N	0 / 5392	-28.0 -28.0	0.25 (1)	10.00
N-I	0 / 5392	-28.0 -28.0	0.25 (1)	10.00
I-H	0 / 5234	-28.0 -28.0	0.25 (1)	10.00
H-O	0 / 5234	-28.0 -28.0	0.25 (1)	10.00
O-P	0 / 5234	-28.0 -28.0	0.25 (1)	10.00
P-G	0 / 5234	-28.0 -28.0	0.25 (1)	10.00
G-Q	0 / 5185	-28.0 -28.0	0.26 (1)	10.00
Q-R	0 / 5185	-28.0 -28.0	0.26 (1)	10.00
R-F	0 / 5185	-28.0 -28.0	0.26 (1)	10.00

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE
J	4-3-4	-1828	-1826	—	BACK
L	2-3-4	-1712	-1712	—	BACK
M	6-3-4	-1826	-1826	—	BACK
N	8-3-4	-1712	-1712	—	BACK
O	10-3-4	-1712	-1712	—	BACK
P	12-3-4	-1712	-1712	—	BACK
Q	14-3-4	-1712	-1712	—	BACK
R	16-3-4	-1714	-1714	—	BACK

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
DL = 3.0	PSF	
BOT CH.	LL = 10.5	PSF
DL = 7.0	PSF	
TOTAL LOAD	= 46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, BOBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.57")
CALCULATED VERT. DEFL.(LL) = L/899 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.57")
CALCULATED VERT. DEFL.(TL) = L/999 (0.09")

CSI: TC=0.07/1.00 (B-C-1), BC=0.27/1.00 (J-K-1), WB=0.47/1.00 (B-K-1), SS=0.50/1.00 (J-I-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

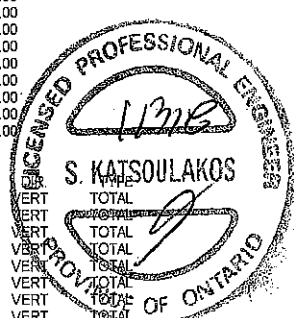
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1657
	822	2284	1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (D) (INPUT = 0.90)
JSI METAL= 0.64 (B) (INPUT = 1.00)



DWG NO. YAH 3340-18
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285748	T36A	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1711.9 lbs FACTORED DOWN AT 2-3-4, 1826.2 lbs FACTORED DOWN AT 4-3-4, 1826.2 lbs FACTORED DOWN AT 6-3-4, 1711.9 lbs FACTORED DOWN AT 8-3-4, 1711.9 lbs FACTORED DOWN AT 10-3-4, 1711.9 lbs FACTORED DOWN AT 12-3-4, AND 1711.9 lbs FACTORED DOWN AT 14-3-4, AND 1714.0 lbs FACTORED DOWN AT 16-3-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.



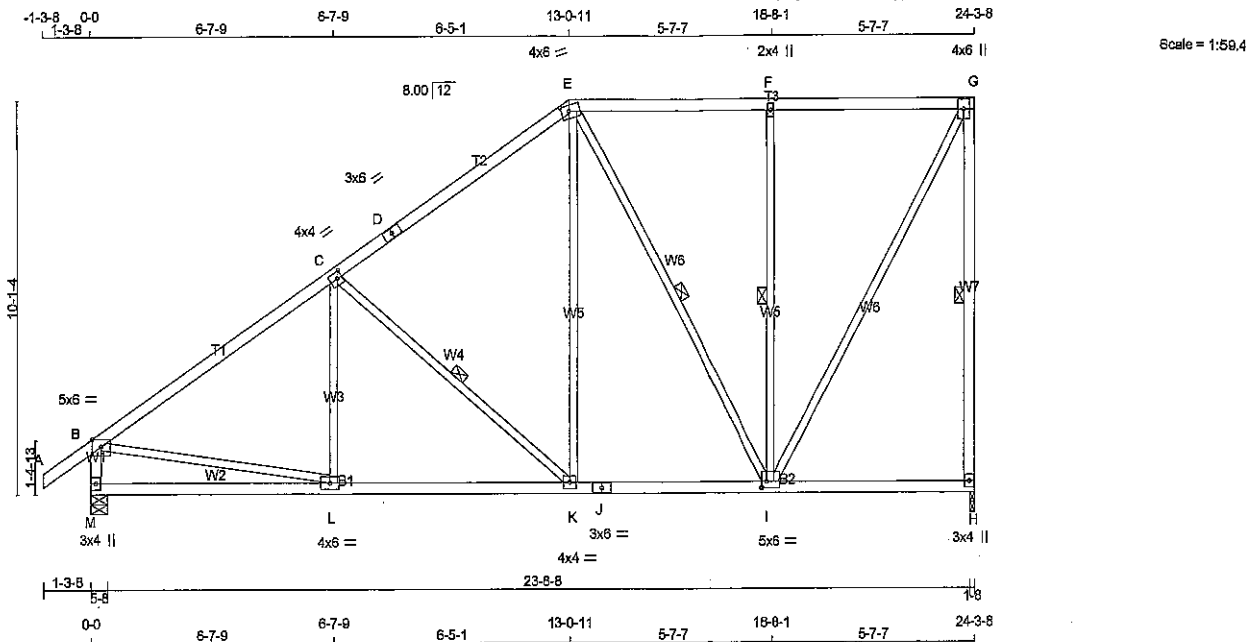
DWONG.TAH 3340-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
286039	T37	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.200 S Dec 12 2017 Mitek Industries, Inc. Tue Jan 16 10:38:29 2018 Page 1

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TOTAL WEIGHT = 2 X 122 = 243 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
H - G	2x4	DRY	No.2	SPF	
M - B	2x4	DRY	No.2	SPF	
M - J	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	4.0	6.0	1.75	2.50
F	TMW+w	MT20	2.0	4.0		
G	TMVW+p	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWWW-t	MT20	5.0	6.0	2.00	1.50
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
H	1363	0	0	1-8
M	1479	0	0	5-8

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1119	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
M	1198	592 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.69 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-H, C-K, E-I, F-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. VERT. LOAD (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. VERT. LOAD (LC)	MAX. VERT. LOAD (LC)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 32	-84.3	-84.3	0.11 (1)	L-C	-1 / 267	0.06 (3)
B-C	-1522 / 0	-84.3	-84.3	0.55 (1)	C-K	-594 / 0	0.30 (1)
C-D	-1051 / 0	-84.3	-84.3	0.49 (1)	K-E	0 / 545	0.12 (2)
D-E	-1051 / 0	-84.3	-84.3	0.49 (1)	E-I	-413 / 0	0.40 (1)
E-F	-644 / 0	-84.3	-84.3	0.34 (1)	I-F	-588 / 0	0.41 (1)
F-G	-844 / 0	-84.3	-84.3	0.34 (1)	I-G	0 / 1289	0.29 (1)
H-G	-1299 / 0	0.0	0.0	0.65 (1)	B-L	0 / 1314	0.30 (1)
M-B	-1404 / 0	0.0	0.0	0.14 (1)			
M-L	0 / 0	-28.0	-28.0	0.31 (3)			
L-K	0 / 1286	-28.0	-28.0	0.48 (2)			
K-J	0 / 848	-28.0	-28.0	0.31 (2)			
J-I	0 / 848	-28.0	-28.0	0.31 (2)			
I-H	0 / 0	-28.0	-28.0	0.21 (3)			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. G.C.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/999 (0.14")

CSI: TC=0.65/1.00 (G-H:1), BC=0.46/1.00 (K-L:2),
WB=0.41/1.00 (F-I:1), SSI=0.23/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

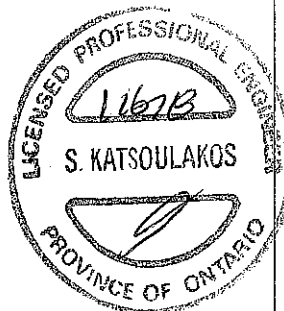
NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1667

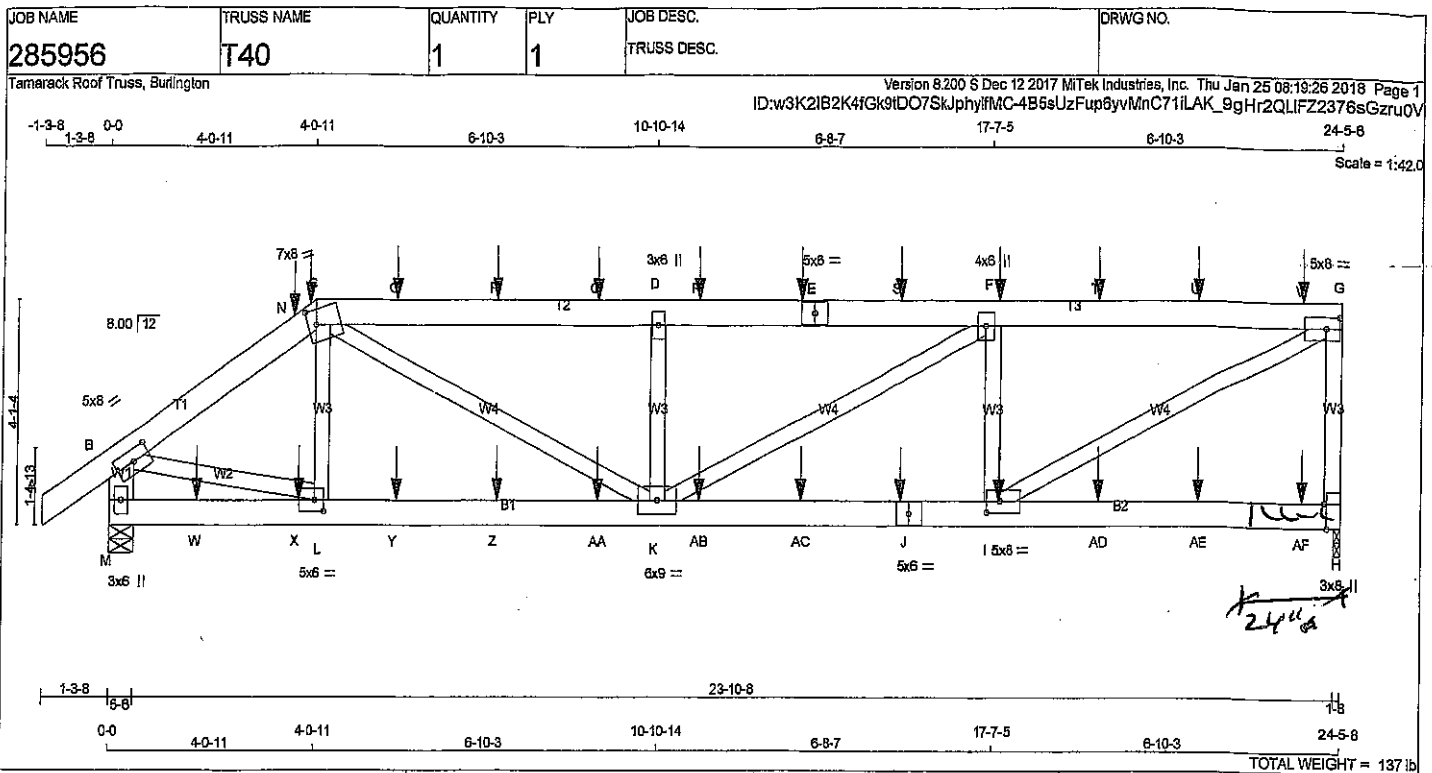
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SSI GRIP= 0.83 (L) (INPUT = 0.90)
SSI METAL= 0.32 (L) (INPUT = 1.00)



DRWG NO. TAM 3894 -18
STRUCTURAL
COMPONENT ONLY



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - G	2x6	DRY	No.2	SPF
H - I	2x4	DRY	No.2	SPF
M - B	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF

ALL WEBS 2x4 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW-t	MT20	5.0	8.0	2.50	3.75
C	TTWW-m	MT20	7.0	8.0	3.25	2.00
D	TMW-w	MT20	3.0	6.0		
E	TS-t	MT20	5.0	6.0		
F	TMW-w	MT20	4.0	6.0		
G	TMW-t	MT20	5.0	8.0	2.50	3.00
H	BMV-t	MT20	3.0	8.0	Edge	0.50
I	BMW-t	MT20	5.0	8.0	2.50	3.00
J	BS-t	MT20	5.0	6.0		
K	BMW-t	MT20	6.0	9.0		
L	BMW-t	MT20	5.0	8.0	2.50	2.25
M	BMV-t	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 244.5 lbs FACTORED DOWN AT 4-0-11, 129.8 lbs FACTORED DOWN AT 3-8-12, 101.4 lbs FACTORED DOWN AT 5-8-12, 101.4 lbs FACTORED DOWN AT 7-8-12, 101.4 lbs FACTORED DOWN AT 9-8-12, 101.4 lbs FACTORED DOWN AT 11-8-12, 101.4 lbs FACTORED DOWN AT 13-8-12, 101.4 lbs FACTORED DOWN AT 15-8-12, 101.4 lbs FACTORED DOWN AT 17-8-12, 101.4 lbs FACTORED DOWN AT 19-8-12, AND 101.4 lbs FACTORED DOWN AT 21-8-12, AND 112.8 lbs FACTORED DOWN AT 23-8-12 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 1-8-12, 69.9 lbs FACTORED DOWN AT 3-8-12, 69.9 lbs FACTORED DOWN AT 5-8-12, 69.9 lbs FACTORED DOWN AT 7-8-12, 69.9 lbs FACTORED DOWN AT 9-8-12, 69.9 lbs FACTORED DOWN AT 11-8-12, 69.9 lbs FACTORED DOWN AT 13-8-12, 69.9 lbs FACTORED DOWN AT 15-8-12, 69.9 lbs FACTORED DOWN AT 17-8-12, 69.9 lbs FACTORED DOWN AT 19-8-12, AND 69.9 lbs FACTORED DOWN AT 21-8-12, AND 76.6 lbs FACTORED DOWN AT 23-8-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		IN-SP	REQD BRG
	VERT	HORZ	DOWN	HORZ		
H	2309	0	2309	0	1-8	5-8
M	2439	0	2439	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	UNBRAC			
H	1907	1040 / 0	447 / 0	0 / 0	0 / 0	420 / 0	0 / 0
M	1992	1134 / 0	429 / 0	0 / 0	0 / 0	420 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.72 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL FITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		MAX. UNBRAC	MEMB.	WEBS		MAX. UNBRAC
	FORCE (LBS)	VERT. LOAD (PLF)	CS (LC)	MAX. CS (LC)			FORCE (LBS)	MAX. CS (LC)	
FR-TO						FR-TO			
A-B	0 / 33	-84.3	-84.3	0.07 (1)	10.00	L-C	-187 / 265	0.05 (3)	
B-N	-2810 / 0	-84.3	-84.3	0.19 (1)	4.80	C-K	0 / 1688	0.33 (1)	
N-C	-2810 / 0	-84.3	-84.3	0.19 (1)	4.80	K-D	-972 / 0	0.17 (1)	
C-O	-4018 / 0	-84.3	-84.3	0.58 (1)	3.72	K-F	0 / 816	0.14 (1)	
O-P	-4018 / 0	-84.3	-84.3	0.58 (1)	3.72	F-G	-1385 / 0	0.24 (1)	
P-Q	-4018 / 0	-84.3	-84.3	0.58 (1)	3.72	I-G	0 / 3739	0.66 (1)	
Q-D	-4018 / 0	-84.3	-84.3	0.58 (1)	3.72	B-L	0 / 2410	0.43 (1)	
D-R	-4018 / 0	-84.3	-84.3	0.58 (1)	3.72				
R-E	-4018 / 0	-84.3	-84.3	0.58 (1)	3.72				
E-S	-4018 / 0	-84.3	-84.3	0.58 (1)	3.72				
S-F	-4018 / 0	-84.3	-84.3	0.58 (1)	3.72				
F-T	-3301 / 0	-84.3	-84.3	0.53 (1)	4.08				
T-U	-3301 / 0	-84.3	-84.3	0.53 (1)	4.08				
U-V	-3301 / 0	-84.3	-84.3	0.53 (1)	4.08				
V-G	-3301 / 0	-84.3	-84.3	0.53 (1)	4.08				
H-G	-2160 / 0	0.0	0.0	0.50 (1)	5.72				
M-B	-2388 / 0	0.0	0.0	0.17 (1)	6.62				
M-W	0 / 0	-28.0	-28.0	0.19 (2)	10.00				
W-X	0 / 0	-28.0	-28.0	0.19 (2)	10.00				
X-L	0 / 0	-28.0	-28.0	0.19 (2)	10.00				
L-Y	0 / 2349	-28.0	-28.0	0.42 (1)	10.00				
Y-Z	0 / 2349	-28.0	-28.0	0.42 (1)	10.00				
Z-AA	0 / 2349	-28.0	-28.0	0.42 (1)	10.00				
AA-K	0 / 2349	-28.0	-28.0	0.42 (1)	10.00				
K-AB	0 / 3301	-28.0	-28.0	0.57 (2)	10.00				
AB-AC	0 / 3301	-28.0	-28.0	0.57 (2)	10.00				
AC-J	0 / 3301	-28.0	-28.0	0.57 (2)	10.00				
J-I	0 / 3301	-28.0	-28.0	0.57 (2)	10.00				
I-AD	0 / 0	-28.0	-28.0	0.26 (3)	10.00				
AD-AE	0 / 0	-28.0	-28.0	0.26 (3)	10.00				
AE-AF	0 / 0	-28.0	-28.0	0.26 (3)	10.00				
AF-H	0 / 0	-28.0	-28.0	0.26 (3)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE
C	4-0-11	-23	-25	---	FRONT	VERT	DEAD
C	4-0-11	-222	-222	---	FRONT	VERT	SNOW
E	13-8-12	-101	-101	---	BACK	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	= 25.8	PSF
	DL	= 3.0	PSF
BOT CH.	LL	= 10.5	PSF
	DL	= 7.0	PSF
TOTAL LOAD	=	48.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

*** NON STANDARD GIRDER ***
ADDITIONAL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 088-09
- TFC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.82")
CALCULATED VERT. DEFL.(LL) = L/999 (0.15")
ALLOWABLE DEFL.(TL) = L/360 (0.82")
CALCULATED VERT. DEFL.(TL) = L/999 (0.24")

CSI: TC=0.58/1.00 (C-D:1), BC=0.57/1.00 (I-K:2), WB=0.66/1.00 (G-I:1), SSI=0.35/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	616	354	1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

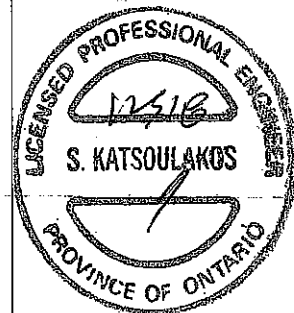
JSI GRIP=0.90 (C) (INPUT = 0.90)
JSI METAL=0.62 (G) (INPUT = 1.00)

DRWG NO. TAM 59-02-18
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2

FACTORED CONCENTRATED LOADS (LBS)						
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.
F	17-8-12	-101	-101	---	BACK	VERT
I	17-8-12	-40	-70	---	BACK	VERT
J	15-8-12	-40	-70	---	BACK	VERT
N	3-8-12	-130	-130	---	BACK	VERT
O	5-8-12	-101	-101	---	BACK	VERT
P	7-8-12	-101	-101	---	BACK	VERT
Q	9-8-12	-101	-101	---	BACK	VERT
R	11-8-12	-101	-101	---	BACK	VERT
S	15-8-12	-101	-101	---	BACK	VERT
T	19-8-12	-101	-101	---	BACK	VERT
U	21-8-12	-101	-101	---	BACK	VERT
V	23-8-12	-113	-113	---	BACK	VERT
W	1-8-12	-40	-70	---	BACK	VERT
X	3-8-12	-40	-70	---	BACK	VERT
Y	5-8-12	-40	-70	---	BACK	VERT
Z	7-8-12	-40	-70	---	BACK	VERT
AA	9-8-12	-40	-70	---	BACK	VERT
AB	11-8-12	-40	-70	---	BACK	VERT
AC	13-8-12	-40	-70	---	BACK	VERT
AD	19-8-12	-40	-70	---	BACK	VERT
AE	21-8-12	-40	-70	---	BACK	VERT
AF	23-8-12	-44	-77	---	BACK	VERT

SCAB JOINT # H WITH 2 X 6 #2
SPF 24" LONG 1 SIDE(S) USING 2
ROW(S) 3/4" COMMON WIRE NAILS
AT 2" O/C STAGGERED (18 NAILS/SCAB)
REQUIRED FOR 1 1/2" BRG PROMSION.

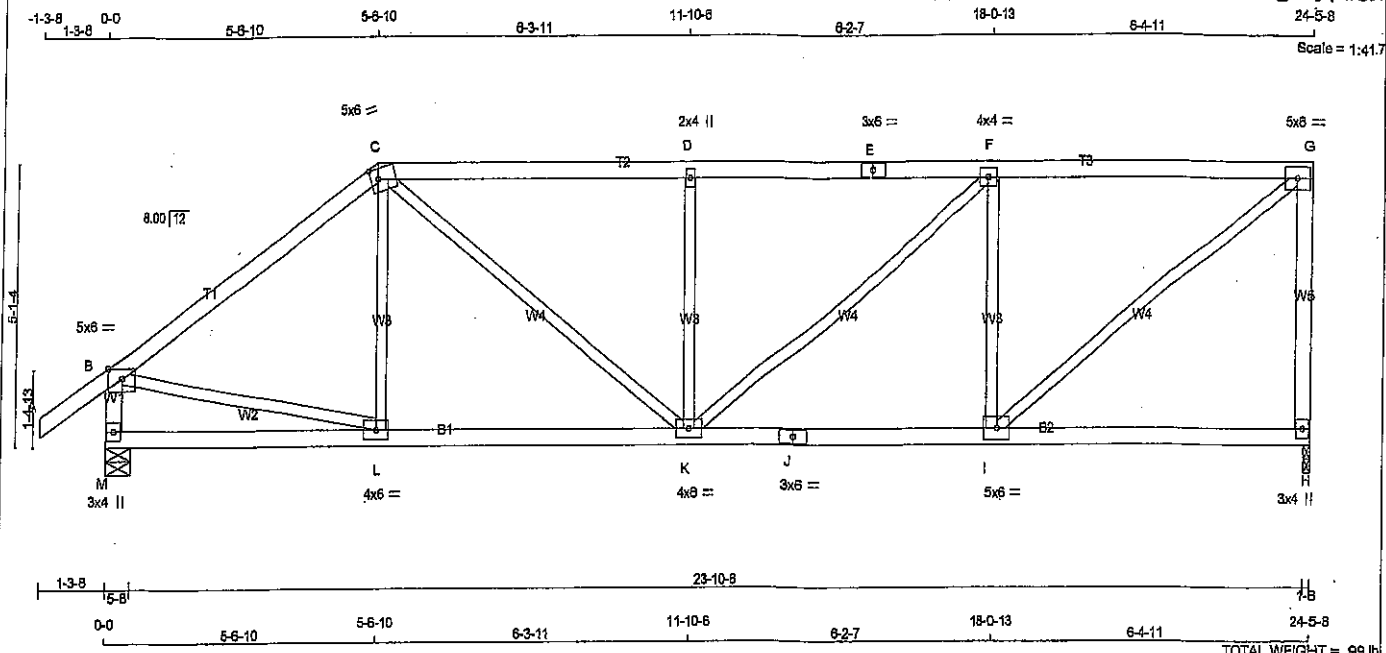


DWG NO. TAM 5902-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285815	T41	1	1	TRUSS DESC.	

Tamerack Roof Truss, Burlington

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ID:w3K2IB2K4fGk9tDO7SkJphylfMC-lQe1GBVaaF8iv8s5KssTXqa8E9?ILW_uudyqzwSo1



LUMBER			
N.L. G.A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
H - G	2x4	DRY	No.2
M - B	2x4	DRY	No.2
M - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW-m	MT20	5.0	6.0	2.25	1.75
D	TMVW-w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMVW-t	MT20	4.0	4.0		
G	TMVW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMVW-t	MT20	5.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMVW-t	MT20	4.0	6.0		
L	BMVW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
H	1373	0	1373	0
M	1488	0	1488	0

UNFACTORED REACTIONS

JT	1ST CASE	MAX	MIN	COMPONENT REACTIONS
	COMBINED	SNOW	LIVE	PERM. LIVE
H	1127	626 / 0	257 / 0	0 / 0
M	1206	696 / 0	257 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.30 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED (LC)	MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED (LC)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 32	-84.3	-84.3	0.11 (1)	L-C	-50 / 204	0.05 (3)
B-C	-1507 / 0	-84.3	-84.3	0.55 (1)	B-L	0 / 1275	0.29 (1)
C-D	-1822 / 0	-84.3	-84.3	0.55 (1)	I-G	0 / 1799	0.40 (1)
D-E	-1822 / 0	-84.3	-84.3	0.55 (1)	C-K	0 / 717	0.16 (1)
E-F	-1822 / 0	-84.3	-84.3	0.55 (1)	I-F	-892 / 0	-0.34 (1)
F-G	-1437 / 0	-84.3	-84.3	0.54 (1)	K-D	-571 / 0	-0.22 (1)
H-G	-1300 / 0	0.0	0.0	0.57 (1)	K-F	0 / 487	0.11 (1)
M-B	-1427 / 0	0.0	0.0	0.15 (1)			
M-L	0 / 0	-28.0	-28.0	0.24 (3)			
L-K	0 / 1251	-28.0	-28.0	0.39 (2)			
K-J	0 / 1437	-28.0	-28.0	0.44 (2)			
J-I	0 / 1437	-28.0	-28.0	0.44 (2)			
I-H	0 / 0	-28.0	-28.0	0.28 (3)			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. G.C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 083-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.82")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.09")
ALLOWABLE DEFL. (TL) = $L/360$ (0.82")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.15")

CSI: TC=0.58/1.00 (D-F:1), BC=0.44/1.00 (I-K:2), WB=0.40/1.00 (G-L:1), SS=0.25/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE	GRIP (DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.59 (K) (INPUT = 0.90)
JSI METAL= 0.44 (J) (INPUT = 1.00)

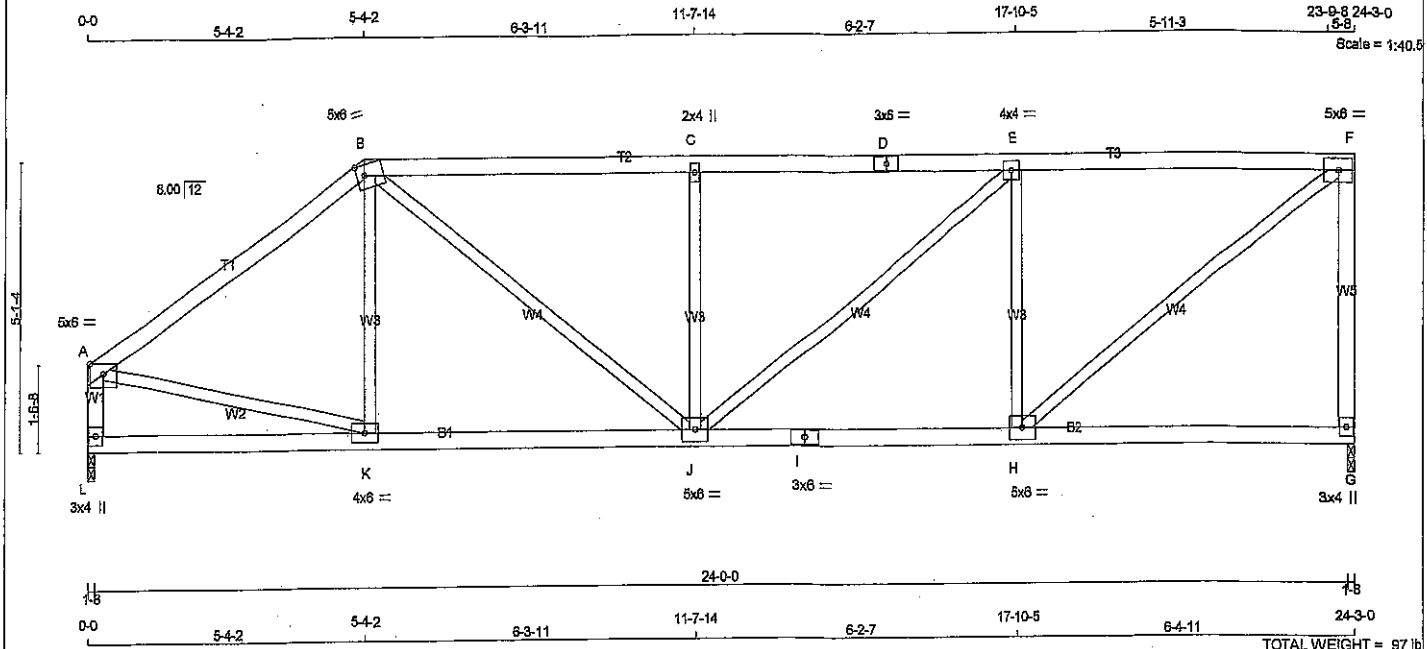


DWNO. TAW3363.18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285815	T41A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:W3K2IB2K4fGk9tDO7SkJphylfMC-Vf767L5nwYLRtgaPGguX6UB6X0vnrTt2etCO9zsY3N



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF	
B - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
L - A	2x4	DRY	No.2	SPF	
L - I	2x4	DRY	No.2	SPF	
I - G	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
A - B	TMVW-p	MT20	5.0	8.0	Edge	
B - D	TTWW-m	MT20	5.0	6.0	2.25	1.75
C - D	TMVW-w	MT20	2.0	4.0		
D - E	TS-1	MT20	3.0	6.0		
E - F	TMVW-t	MT20	4.0	4.0		
F - G	TMVW-t	MT20	5.0	6.0		
G - H	BMV1+p	MT20	3.0	4.0		
H - I	BMVW-t	MT20	5.0	6.0		
I - J	BS-1	MT20	3.0	6.0		
J - K	BMVWW-t	MT20	5.0	8.0		
K - L	BMVW-t	MT20	4.0	6.0		
L - G	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	DOWN	HORZ	DOWN	HORZ	UP/LIFT	IN-SX	BRG	IN-SX
G	1361	0	0	1361	0	0	1-8	1-8	
L	1361	0	0	1361	0	0	1-8	1-8	

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1118	620 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0	0 / 0
L	1118	620 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.33 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. MEMB. FORCE (LBS)
FR-TO		FROM TO	LENGTH FR-TO
A-B	-1451 / 0	-84.3 -84.3 0.50 (1)	4.82
B-C	-1791 / 0	-84.3 -84.3 0.56 (1)	4.38
C-D	-1792 / 0	-84.3 -84.3 0.57 (1)	4.33
D-E	-1792 / 0	-84.3 -84.3 0.57 (1)	4.33
E-F	-1422 / 0	-84.3 -84.3 0.54 (1)	4.79
G-F	-1289 / 0	0.0 0.0 0.57 (1)	7.10
L-A	-1302 / 0	0.0 0.0 0.14 (1)	7.08
L-K	0 / 0	-28.0 -28.0 0.23 (3)	10.00
K-J	0 / 1204	-28.0 -28.0 0.38 (2)	10.00
J-I	0 / 1422	-28.0 -28.0 0.44 (2)	10.00
I-H	0 / 1422	-28.0 -28.0 0.44 (2)	10.00
H-G	0 / 0	-28.0 -28.0 0.28 (3)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 240 IN./G

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/999 (0.09")
ALLOWABLE DEFL.(TL) = L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/999 (0.14")

CSI: TC=0.57/1.00 (C-E-1), BC=0.44/1.00 (H-J-2), WB=0.40/1.00 (F-H-1), SS=0.25/1.00 (E-F-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 615 354 1667 822 2284 1056

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (K) (INPUT = 0.90)
JSI METAL= 0.44 (I) (INPUT = 1.00)



DRWG NO. TAM 5072-18
STRUCTURAL
COMPONENT ONLY

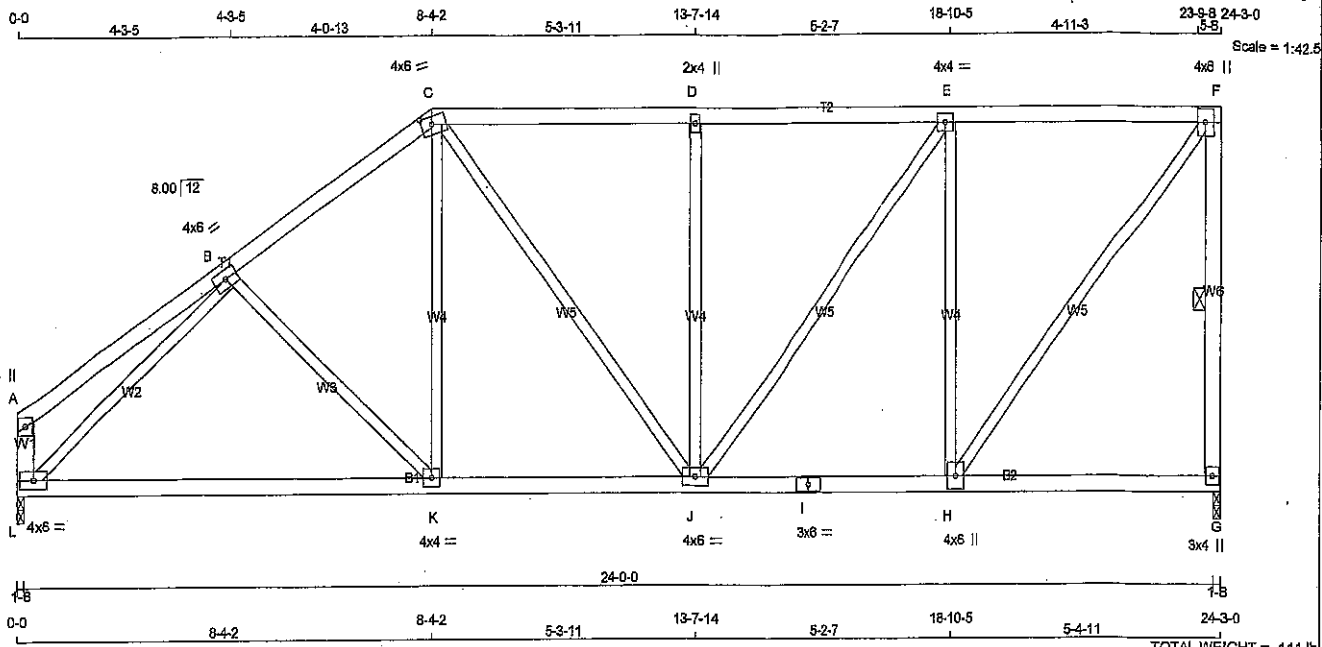
DWG NO. TAM 5073 -18
STRUCTURAL
COMPONENT ONLY

JOB NAME 285815	TRUSS NAME T43A	QUANTITY 1	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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LUMBER	N L G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - F	2x4	DRY	No.2	SPF	
G - I	2x4	DRY	No.2	SPF	
L - A	2x4	DRY	No.2	SPF	
L - I	2x4	DRY	No.2	SPF	
I - G	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMVW-t	MT20	4.0	6.0		
C	TTVW-t	MT20	4.0	6.0	1.75	2.50
D	TMVW-w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV+p	MT20	3.0	4.0		
H	BMVW-t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	8.0		
J	BMVW-t	MT20	4.0	6.0		
K	BMVW-t	MT20	4.0	4.0		
L	BMVW-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED		MAXIMUM FACTORED			INPUT	REQRD
	GROSS REACTION		GROSS REACTION			BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1361	0	1361	0	0	1-8	1-8
L	1361	0	1361	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LOASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1118	620 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
L	1118	620 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)	UNBRACED LENGTH (FR-TO)
A-B	0 / 22	-84.3	-84.3 0.23 (1)	10.00	B-K	-77 / 99	0.05 (1)
B-C	-1369 / 0	-84.3	-84.3 0.27 (1)	5.25	K-C	0 / 367	0.09 (2)
C-D	-1227 / 0	-84.3	-84.3 0.35 (1)	5.36	L-B	-1629 / 0	0.90 (1)
D-E	-1227 / 0	-84.3	-84.3 0.38 (1)	5.33	H-F	0 / 1425	0.32 (1)
E-F	-884 / 0	-84.3	-84.3 0.34 (1)	6.05	C-J	0 / 168	0.04 (1)
F-G	-1302 / 0	0.0	0.0 0.30 (1)	5.66	H-E	-940 / 0	0.82 (1)
L-A	-136 / 0	0.0	0.0 0.01 (1)	7.81	J-D	-479 / 0	0.42 (1)
L-K	0 / 1175	-28.0	-28.0 0.55 (2)	10.00	J-E	0 / 565	0.13 (1)
K-J	0 / 1124	-28.0	-28.0 0.57 (2)	10.00			
J-I	0 / 894	-28.0	-28.0 0.32 (2)	10.00			
I-H	0 / 894	-28.0	-28.0 0.32 (2)	10.00			
H-G	0 / 0	-28.0	-28.0 0.21 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 3.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2012, SCBC 2012, ABC 2014
- CSA C88-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL)= L/899 (0.21")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL)= L/853 (0.34")

CSI: TC=0.36/1.00 (D-E-1), BC=0.57/1.00 (J-K-2),
WB=0.80/1.00 (B-L-1), SSI=0.21/1.00 (E-F-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354
	1657	822
	2284	1655

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.40 (B) (INPUT = 1.00)



DRWG NO. TAM 5074-8
STRUCTURAL
COMPONENT ONLY

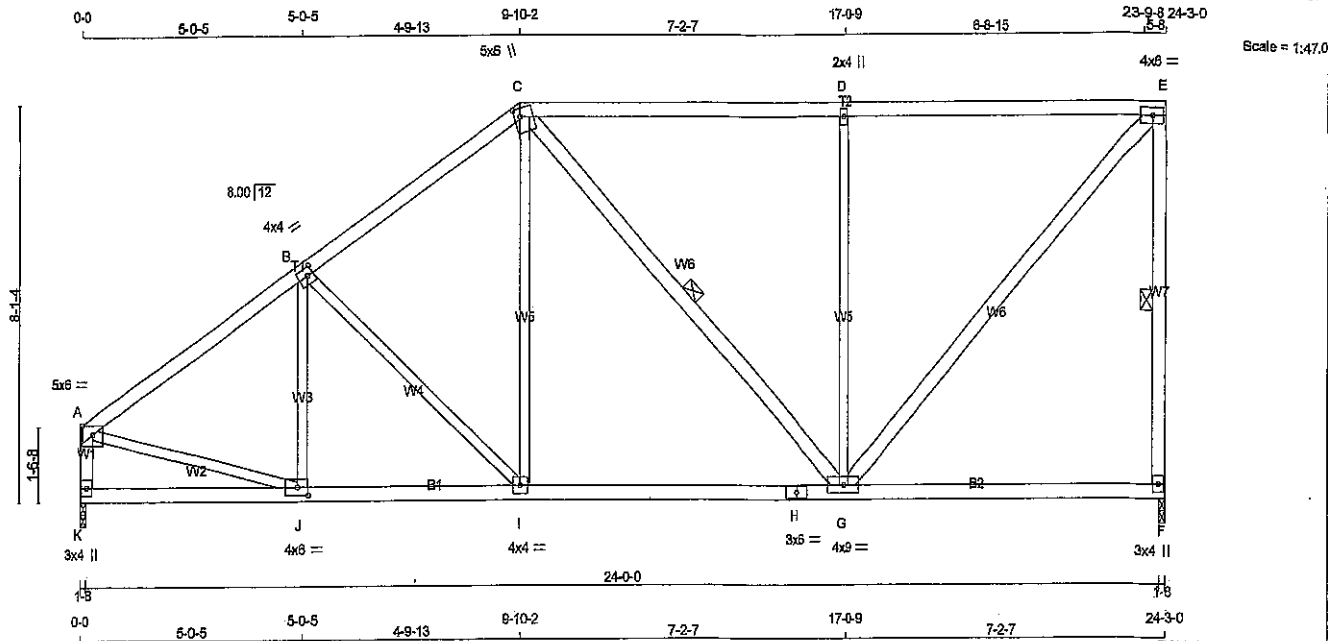
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285815	T44A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:w3K2IB2K4fGk9DO7SkJphlyfMC-zrhUKh5Th2gC31Fnz_B73K1IrwLFaE30HidwczsY3M

23-9-8 24-3-0

Scale = 1:47.0



LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
C - G	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW-p	MT20	5.0	6.0	Edge
B	TMVW-t	MT20	4.0	4.0	2.00 1.50
C	TTWW+m	MT20	5.0	6.0	2.00 1.50
D	TMVW-w	MT20	2.0	4.0	
E	TMVW-t	MT20	4.0	6.0	
F	BMV1+p	MT20	3.0	4.0	
G	BMVWW-t	MT20	4.0	9.0	
H	BS-t	MT20	3.0	6.0	
I	BMVW-t	MT20	4.0	4.0	
J	BMVW-t	MT20	4.0	6.0	2.00 2.75
K	BMV1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS							
	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG			
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	1361	0	1361	0	0	1-8	1-8
K	1361	0	1361	0	0	1-8	1-8
UNFACTORED REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1118	620 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
K	1118	620 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.74 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-F, C-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MAX. FACTORED		FACTORED			MAX. FACTORED				
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX. CS1 (LC)	MAX. UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM TO				FR-TO			
A-B	-1477 / 0	-84.3	-84.3	0.40 (1)	4.93	J-B	-158 / 110	0.06 (1)	
B-C	-1264 / 0	-84.3	-84.3	0.38 (1)	5.24	B-I	-307 / 0	0.26 (1)	
C-D	-973 / 0	-84.3	-84.3	0.83 (1)	4.74	I-C	0 / 419	0.09 (2)	
D-E	-973 / 0	-84.3	-84.3	0.83 (1)	4.74	C-G	-87 / 0	0.05 (1)	
E-F	-1281 / 0	0.0	0.0	0.38 (1)	5.70	G-D	-754 / 0	0.95 (1)	
K-A	-1301 / 0	0.0	0.0	0.14 (1)	7.08	G-E	0 / 1431	0.23 (1)	
						A-J	0 / 1288	0.29 (1)	
K-J	0 / 0	-28.0	-28.0	0.15 (3)	10.00				
J-I	0 / 1252	-28.0	-28.0	0.33 (2)	10.00				
I-H	0 / 1032	-28.0	-28.0	0.50 (2)	10.00				
H-G	0 / 1032	-28.0	-28.0	0.50 (2)	10.00				
G-F	0 / 0	-28.0	-28.0	0.38 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.81")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.10")
ALLOWABLE DEFL.(TL) = $L/360$ (0.81")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.17")

CS1: TC=0.83/1.00 (D-E-1), BC=0.50/1.00 (G-I-2), WB=0.93/1.00 (D-G-1), SS1=0.30/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1687 822	2284 1856

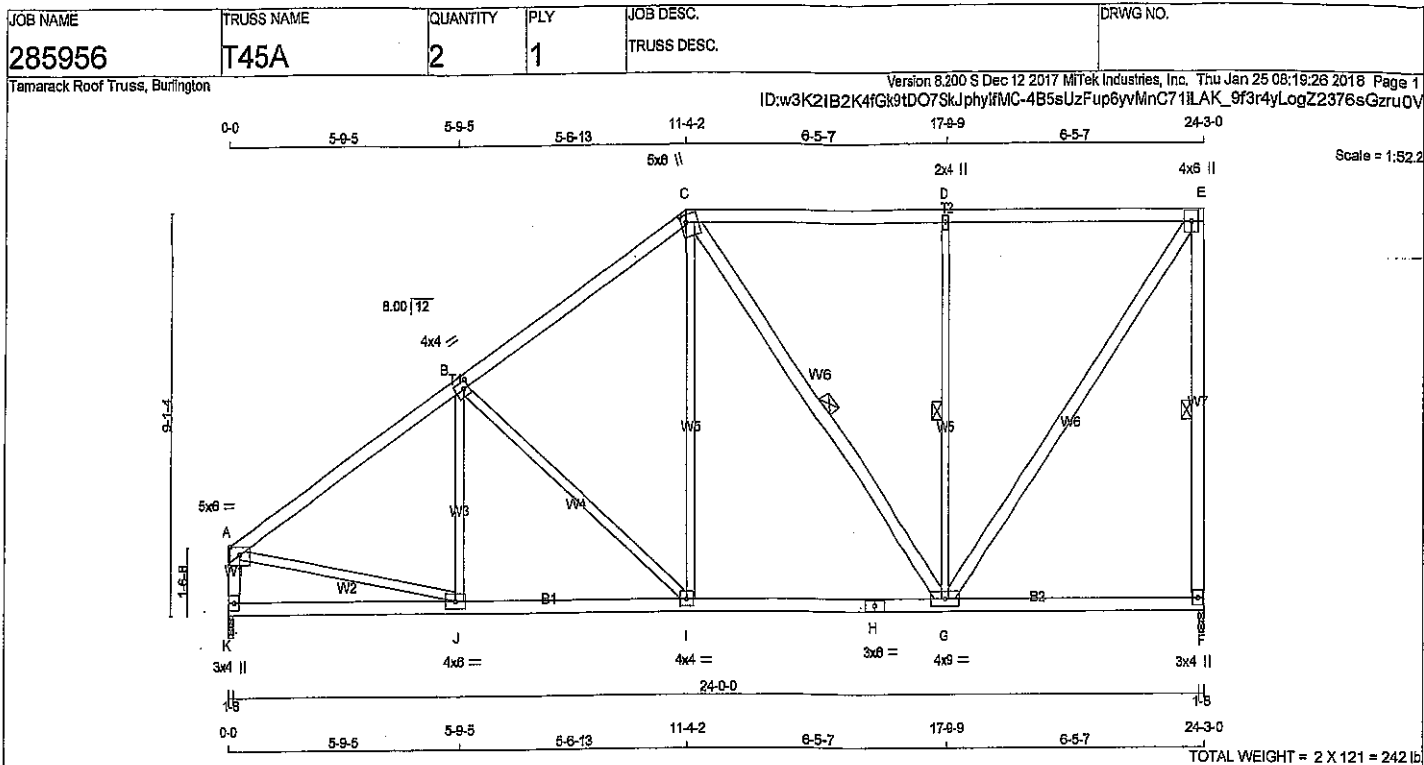
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (E) (INPUT = 0.90)
JSI METAL= 0.41 (H) (INPUT = 1.00)



DRWG NO. TAM 5075 -18
STRUCTURAL
COMPONENT ONLY



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
F - E	2x4	DRY	No.2
K - A	2x4	DRY	No.2
K - H	2x4	DRY	No.2
H - F	2x4	DRY	No.2

ALL WEBS EXCEPT			
C - G	2x4	DRY	No.2
G - E	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
A	TMWV-p	MT20	5.0	8.0 Edge
B	TMWV-t	MT20	4.0	4.0 2.00 1.50
C	TTWV+m	MT20	5.0	8.0 2.00 1.50
D	TMWV+w	MT20	2.0	4.0
E	TMWV+p	MT20	4.0	8.0
F	BMV1+p	MT20	3.0	4.0
G	BMWVW-t	MT20	4.0	8.0
H	BS-t	MT20	3.0	6.0
I	BMWVW-t	MT20	4.0	4.0
J	BMWVW-t	MT20	4.0	6.0
K	BMV1+p	MT20	3.0	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
F	1361	0	1361	0	0	1-8	1-8	1-8	1-8
K	1361	0	1361	0	0	1-8	1-8	1-8	1-8

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN. COMPONENT REACTIONS		LIVE		PERM. LIVE		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	SNOW	SNOW	SNOW	SNOW	SNOW	SNOW	SNOW	SNOW	SNOW	SNOW	SNOW
F	1118	620 / 0	255 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	242 / 0	0 / 0	0 / 0	0 / 0
K	1118	620 / 0	255 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	242 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.70 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-F, C-G, D-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	MAX. FORCE (LBS)	VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FORCE (LBS)	VERT. LOAD (PLF)	MAX. CS1 (LC)
FR-TO				FR-TO			
A-B	-1488 / 0	-84.3	-84.3 0.56 (1)	J-B	-89 / 183	0.04 (3)	
B-C	-1181 / 0	-84.3	-84.3 0.52 (1)	B-I	-435 / 0	0.51 (1)	
C-D	-796 / 0	-84.3	-84.3 0.63 (1)	I-C	0 / 475	0.11 (2)	
D-E	-796 / 0	-84.3	-84.3 0.63 (1)	C-G	-249 / 0	0.16 (1)	
F-E	-1288 / 0	0.0	0.0 0.50 (1)	G-D	-675 / 0	0.35 (1)	
K-A	-1285 / 0	0.0	0.0 0.14 (1)	G-E	0 / 1343	0.22 (1)	
				A-J	0 / 1290	0.29 (1)	
K-J	0 / 0	-28.0	-28.0 0.22 (3)				
J-I	0 / 1262	-28.0	-28.0 0.37 (2)				
I-H	0 / 943	-28.0	-28.0 0.41 (2)				
H-G	0 / 943	-28.0	-28.0 0.41 (2)				
G-F	0 / 0	-28.0	-28.0 0.30 (3)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/999 (0.12")

CS1: TC=0.83/1.00 (D-E-1), BC=0.41/1.00 (G-I-2), WB=0.51/1.00 (B-I-1), SS=0.28/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

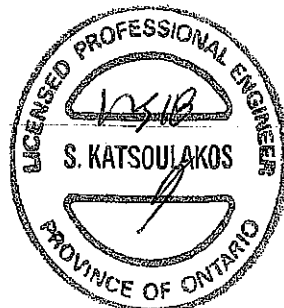
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 510 354 1667 822 2284 1656

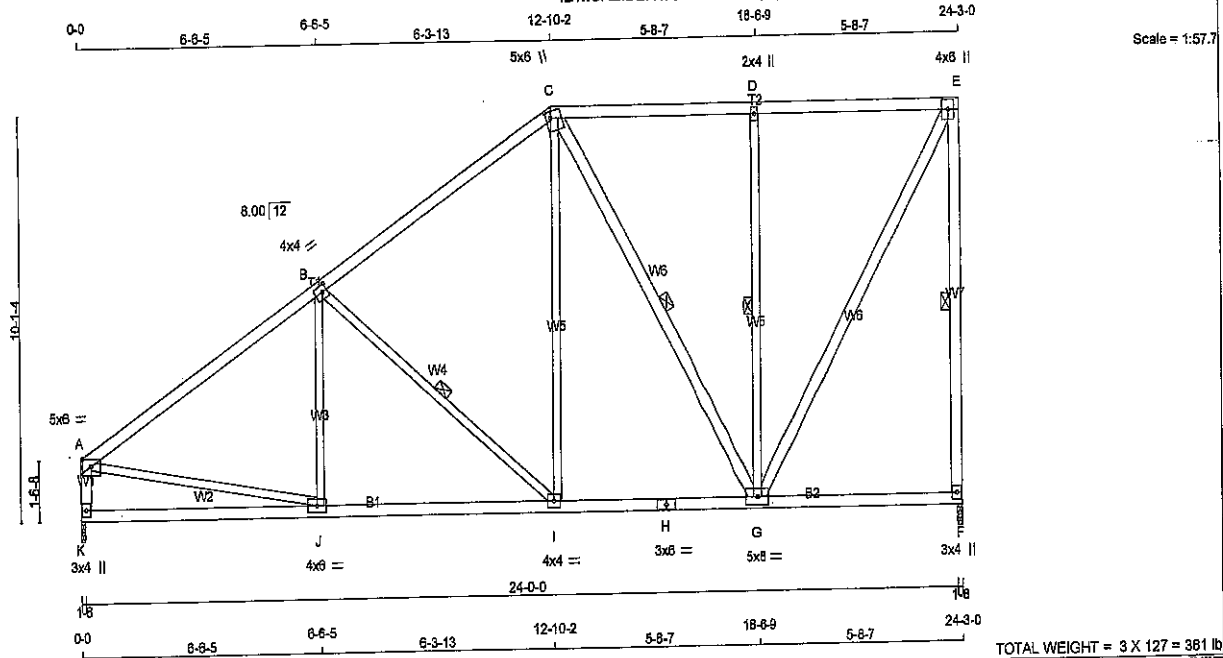
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (G) (INPUT = 0.90)
JSI METAL= 0.32 (J) (INPUT = 1.00)



DRWG NO. TAM 5984-8
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS	2x6	DRY	No.2	SPF
EXCEPT				
C - G	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	6.0	Edge	
B	TMVW-t	MT20	4.0	4.0	2.00	1.50
C	TMVW+m	MT20	5.0	6.0	2.00	1.50
D	TMVW+w	MT20	2.0	4.0		
E	TMVW+p	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMVWVW-t	MT20	5.0	8.0		
H	BS-t	MT20	3.0	6.0		
I	BMVW-t	MT20	4.0	4.0		
J	BMVW-t	MT20	4.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
F	1361	0	1361	0
K	1361	0	1361	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN	COMPONENT REACTIONS	WIND	DEAD	SOIL
F	1118	620 / 0	255 / 0	0 / 0	243 / 0	0 / 0
K	1118	820 / 0	255 / 0	0 / 0	243 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-F, B-I, C-G, D-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM TO		FR-TO			
A-B	-1481 / 0	-84.3	-84.3 0.52 (1)	J-B	-29 / 248	0.08 (3)	
B-C	-1050 / 0	-84.3	-84.3 0.48 (1)	B-I	-557 / 0	0.27 (1)	
C-D	-651 / 0	-84.3	-84.3 0.35 (1)	I-C	0 / 531	0.12 (2)	
D-E	-651 / 0	-84.3	-84.3 0.35 (1)	C-G	-393 / 0	0.28 (1)	
F-E	-1286 / 0	0.0	0.0 0.65 (1)	G-D	-595 / 0	0.42 (1)	
K-A	-1288 / 0	0.0	0.0 0.13 (1)	G-E	0 / 1288	0.21 (1)	
				A-J	0 / 1284	0.29 (1)	
K-J	0 / 0	-28.0	-28.0 0.30 (3)				
J-I	0 / 1262	-28.0	-28.0 0.44 (2)				
I-H	0 / 847	-28.0	-28.0 0.32 (2)				
H-G	0 / 847	-28.0	-28.0 0.32 (2)				
G-F	0 / 0	-28.0	-28.0 0.22 (3)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 8 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/999 (0.13")

CS1: TC=0.85/1.00 (E-F:1), BC=0.44/1.00 (I-J:2), WB=0.42/1.00 (D-G:1), SS1=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (J) (INPUT = 0.90)
JSI METAL= 0.32 (J) (INPUT = 1.00)



DRWG NO. TAM 5985-18
STRUCTURAL
COMPONENT ONLY

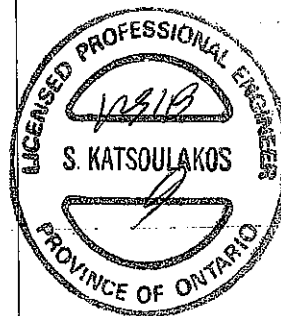
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285956	T47	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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FACTORED CONCENTRATED LOADS (LBS)						
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.
Y	5-7-12	-40	-70	—	FRONT	VERT
Z	7-7-12	-40	-70	—	FRONT	VERT
AA	9-7-0	-863	-963	—	FRONT	VERT
						TYPE
						TOTAL
						TOTAL
						TOTAL

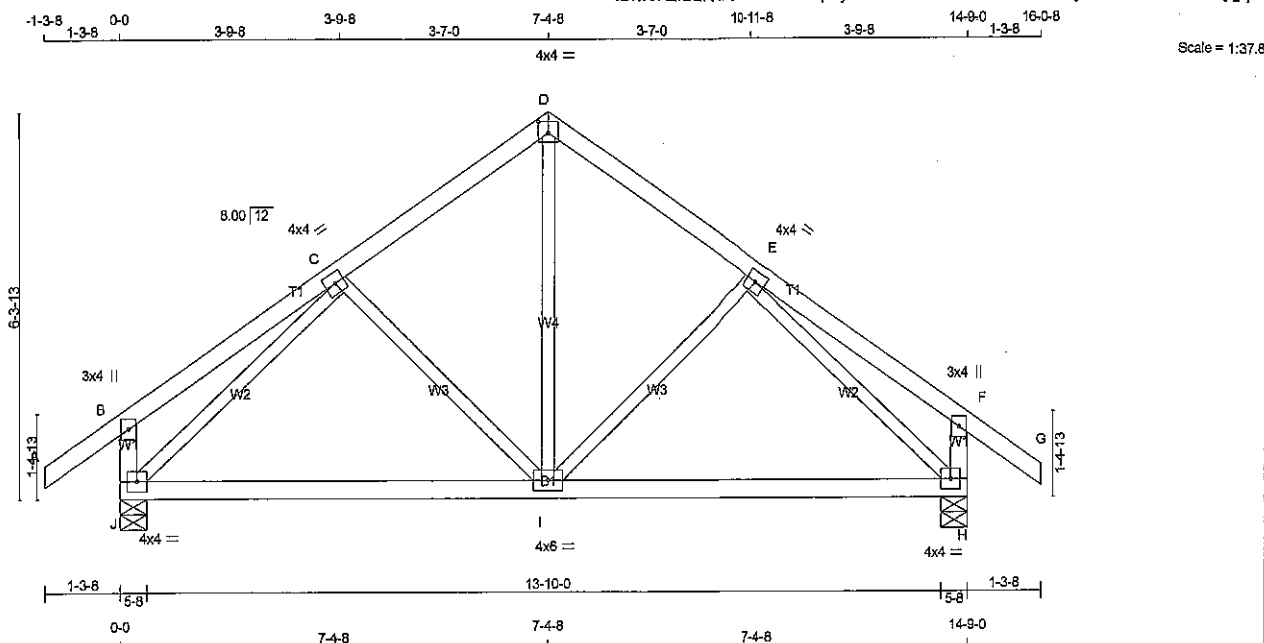


DWG NO. TAM 5986
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43854	DRWG NO.
285745	T48	3	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 3 X 63 = 190 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMVW1-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REGRD BRG
	VERT	HORZ	DOWN	HORZ		
J	944	0	944	0	5-8	5-8
H	944	0	944	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX / MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	WIND			
J	758	448 / 0	155 / 0	0 / 0	138 / 0	0 / 0	0 / 0
H	758	448 / 0	155 / 0	0 / 0	138 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		WEBS		FACTORED	
	MAX. FORCE (LBS)	VERT. LOAD (PLF)	MAX. VERT. LOAD (LBS)	MAX. HORIZ. LOAD (LBS)	MEMB. FORCE (LBS)	MAX. HORIZ. LOAD (LBS)	MAX. VERT. LOAD (LBS)	MAX. HORIZ. LOAD (LBS)
FR-TO					FR-TO			
A-B	0 / 32	-84.3	-84.3	0.11 (1)	I-D	0 / 478	0.11 (1)	
B-C	0 / 19	-84.3	-84.3	0.18 (1)	I-E	-161 / 43	0.07 (1)	
C-D	-868 / 0	-84.3	-84.3	0.14 (1)	C-I	-161 / 44	0.07 (1)	
D-E	-868 / 0	-84.3	-84.3	0.14 (1)	J-C	-909 / 0	0.38 (1)	
E-F	0 / 19	-84.3	-84.3	0.18 (1)	E-H	-909 / 0	0.38 (1)	
F-G	0 / 32	-84.3	-84.3	0.11 (1)				
J-B	-235 / 0	0.0	0.0	0.02 (1)				
H-F	-235 / 0	0.0	0.0	0.02 (1)				
J-I	0 / 855	-28.0	-28.0	0.52 (2)				
I-H	0 / 855	-28.0	-28.0	0.52 (2)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.49")
CALCULATED VERT. DEFL.(LL)= L/999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.49")
CALCULATED VERT. DEFL.(TL)= L/999 (0.15")

CSI: TC=0.18 (B-C:1), BC=0.52 (I-J:2), WB=0.38 (C-J:1), SS=0.18 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	818	354	1667
	622	2284	1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (D) (INPUT = 0.90)
JSI METAL= 0.32 (C) (INPUT = 1.00)



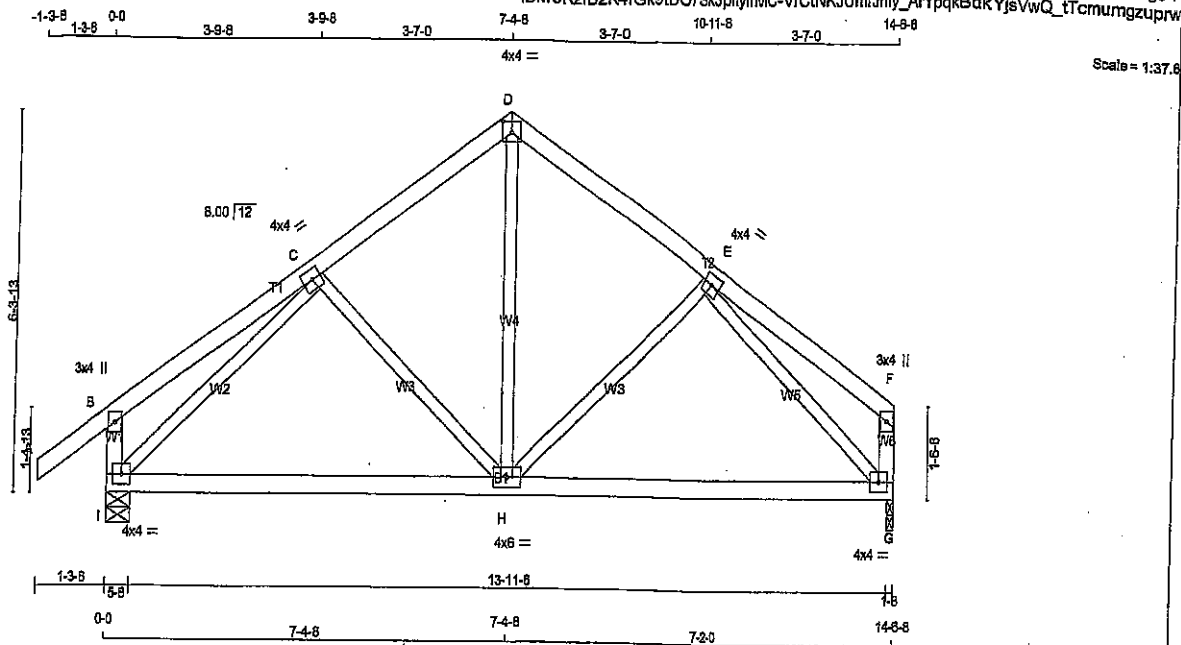
DRWG NO. TAM 4493217
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
285814	T48A	6	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:w3K2IB2K4fGk9tD07SkJphyfMC-VrCtNKJUmJhfy_ArYpqkBdkYjsVwQ_tTcmumgzuprw



TOTAL WEIGHT = 6 X 61 = 366 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4 DRY	No.2	SPF		
D - F	2x4 DRY	No.2	SPF		
I - B	2x4 DRY	No.2	SPF		
G - F	2x4 DRY	No.2	SPF		
I - G	2x4 DRY	No.2	SPF		

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	4.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMVW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

MEMB.	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
JT	832	0	832	0	5-8	5-8
I	832	0	832	0	1-8	1-8
G	816	0	816	0	1-8	1-8

UNFACTORED REACTIONS

MEMB.	1ST LCASE		MAX/MIN. COMPONENT REACTIONS		DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE		
JT	749	442 / 0	153 / 0	0 / 0	0 / 0	0 / 0
I	749	442 / 0	153 / 0	0 / 0	154 / 0	0 / 0
G	670	372 / 0	153 / 0	0 / 0	145 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. FURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		MAX. UNBRACED LENGTH	MEMB.	WEBS	
	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (PLF)	LC1 MAX (PLF)			MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH
FR-TO						FR-TO		
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	H-D	0 / 455	0.10 (1)
B-C	0 / 20	-84.3	-84.3	0.18 (1)	10.00	H-E	-130 / 53	0.06 (1)
C-D	-649 / 0	-84.3	-84.3	0.14 (1)	6.25	C-H	-164 / 42	0.07 (1)
D-E	-848 / 0	-84.3	-84.3	0.13 (1)	8.25	I-C	-881 / 0	0.37 (1)
E-F	0 / 20	-84.3	-84.3	0.17 (1)	10.00	E-G	-882 / 0	0.34 (1)
I-B	-235 / 0	0.0	0.0	0.02 (1)	7.81			
G-F	-111 / 0	0.0	0.0	0.01 (1)	7.81			
I-H	0 / 643	-28.0	-28.0	0.50 (2)	10.00			
H-G	0 / 618	-28.0	-28.0	0.50 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.8 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2012, CBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.48")
CALCULATED VERT. DEFL. (LL) = L/999 (0.09")
ALLOWABLE DEFL. (TL) = L/360 (0.48")
CALCULATED VERT. DEFL. (TL) = L/999 (0.15")

CSI: TC=0.18/1.00 (B-C:1), BC=0.50/1.00 (H-I:2),
WB=0.37/1.00 (C-I:1), SS=0.17/1.00 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

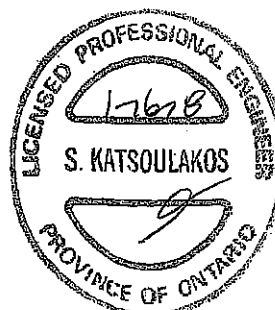
PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 394	1687 822
	2284	1855

PLATE PLACEMENT TOL. = 0.250 inches

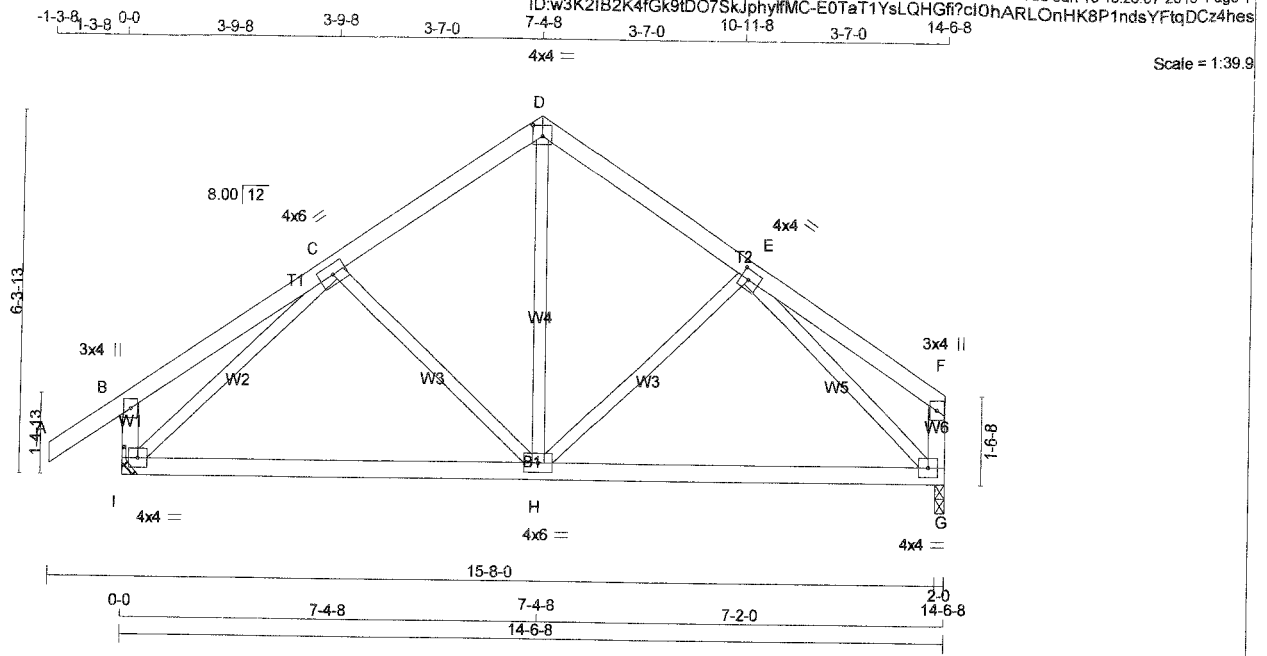
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (E) (INPUT = 0.90)

JSI METAL= 0.32 (C) (INPUT = 1.00)



DWG NO. TAM 3983-18
STRUCTURAL
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LUMBER				
N. L. G. A. RULES				
CHORDS		SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0	2.00	1.75
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BMWWV-t	MT20	4.0	6.0		
I	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
G	816	0	816	0	2-0	2-0
I	932	0	932	0	MECHANICAL	

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT I.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM.LIVE	LIVE			
G	670	372 / 0	153 / 0	0 / 0	0 / 0	145 / 0	0 / 0
I	749	442 / 0	153 / 0	0 / 0	0 / 0	154 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CS1 (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	FACTORED FORCE (LBS)	MAX CS1 (LC)		
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	H-D	0 / 456		
B-C	0 / 20	-84.3	-84.3	0.18 (1)	10.00	H-E	-130 / 53		
C-D	-649 / 0	-84.3	-84.3	0.14 (1)	6.25	C-H	-164 / 41		
D-E	-648 / 0	-84.3	-84.3	0.13 (1)	6.25	I-C	-891 / 0		
E-F	0 / 19	-84.3	-84.3	0.17 (1)	10.00	E-G	-882 / 0		
I-B	-236 / 0	0.0	0.0	0.02 (1)	7.81				
G-F	-111 / 0	0.0	0.0	0.01 (1)	7.81				
I-H	0 / 643	-28.0	-28.0	0.50 (2)	10.00				
H-G	0 / 618	-28.0	-28.0	0.50 (2)	10.00				

TOTAL WEIGHT = 3 X 61 = 183 lb [MJP]

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.48")
CALCULATED VERT. DEFL.(LL)= L/999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.48")
CALCULATED VERT. DEFL.(TL)= L/999 (0.15")

CSI: TC=0.18/1.00 (B-C:1), BC=0.50/1.00 (H-I:2), WB=0.37/1.00 (C-I:1), SSI=0.17/1.00 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

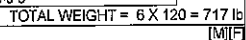
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MT20	618	354	1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (E) (INPUT = 0.90)
JSI METAL= 0.31 (E) (INPUT = 1.00)





DWG NO. TAM 44932-17
STRUCTURAL
COMPONENT ONLY

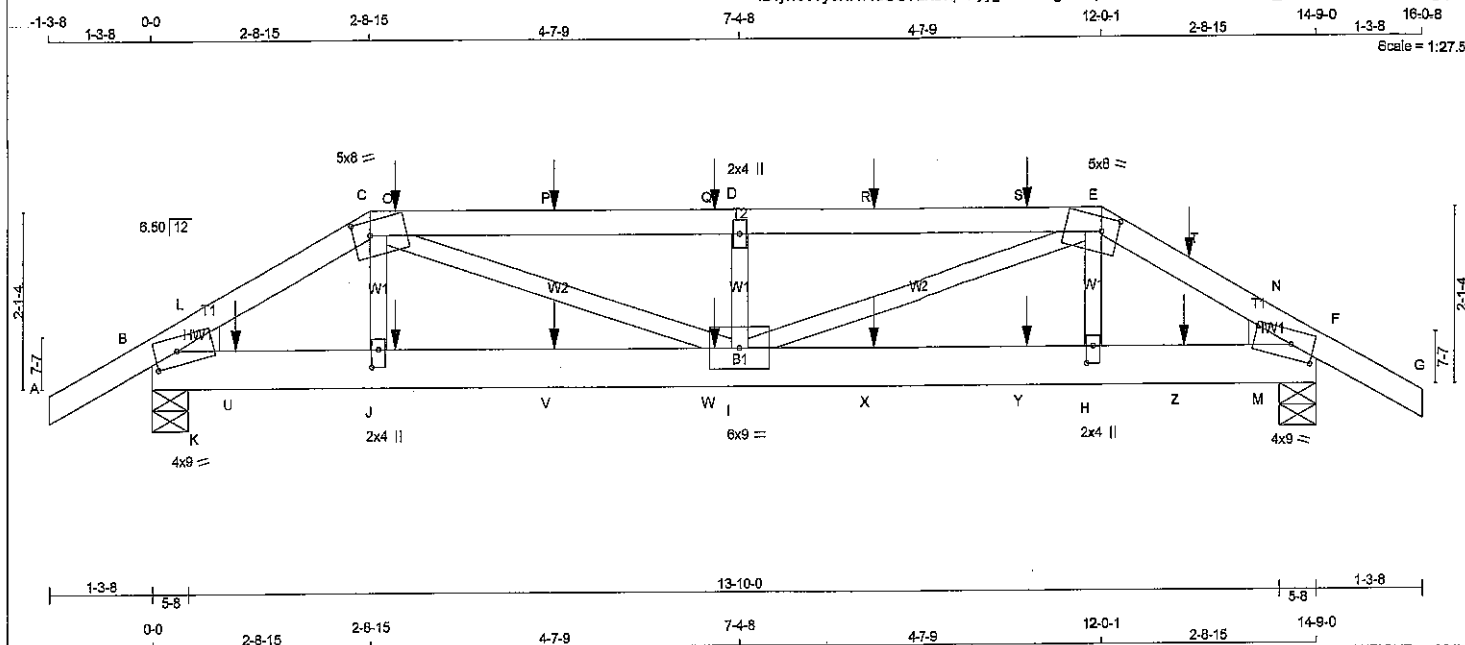
JSI GRIP= 0.90 (O) (INPUT = 0.90)
JSI METAL= 0.69 (L) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
285745	T80	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID: yh8Wy9x7RuUdYExBfHxygInM-Vug4uOpMNBm1HnxURQxCzk9_uIRDNjO2hpH2n2ygipS



TOTAL WEIGHT = 80 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
B - F	2x6	DRY	No.2
ALL WEBS 2x3 DRY			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMBH1-m	MT20	4.0	9.0	2.00 3.50
C	TTWW-m	MT20	5.0	8.0	2.00 2.50
D	TWW+w	MT20	2.0	4.0	
E	TTWW-m	MT20	5.0	8.0	2.00 2.50
F	TMBH1-m	MT20	4.0	9.0	2.00 3.50
H	BMW+w	MT20	2.0	4.0	2.50 1.00
I	BMWWW-t	MT20	6.0	9.0	
J	BMW+w	MT20	2.0	4.0	2.50 1.00

HANGERS NOTES
 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 144.6 lbs FACTORED DOWN AT 3-0-12, 115.9 lbs FACTORED DOWN AT 5-0-12, 115.9 lbs FACTORED DOWN AT 7-0-12, 115.9 lbs FACTORED DOWN AT 9-0-12, AND 118.3 lbs FACTORED DOWN AT 11-0-12, AND 115.9 lbs FACTORED DOWN AT 13-0-12 ON TOP CHORD, AND 76.0 lbs FACTORED DOWN AT 1-0-12, 76.0 lbs FACTORED DOWN AT 3-0-12, 76.0 lbs FACTORED DOWN AT 5-0-12, 76.0 lbs FACTORED DOWN AT 7-0-12, AND 76.0 lbs FACTORED DOWN AT 9-0-12, AND 76.0 lbs FACTORED DOWN AT 11-0-12, AND 76.0 lbs FACTORED DOWN AT 13-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD		HEEL	
JT	VERT	GROSS REACTION	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	WEDGE		
B	1542	0	1542	0	0	5-8	5-8	5-8	2x4 L		
F	1568	0	1568	0	0	5-8	5-8	5-8	2x4 R		

UNFACTORED REACTIONS						
JT	1ST CASE	MAX/MIN	COMPONENT	REACTIONS	DEAD	SOIL
B	1256	714 / 0	275 / 0	0 / 0	0 / 0	0 / 0
F	1271	733 / 0	271 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.02 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 20	-84.3 -84.3	0.12 (1)	J-C	0 / 291	0.07 (2)	
B-L	-2583 / 0	-84.3 -84.3	0.20 (1)	C-I	0 / 1410	0.35 (1)	
L-C	-2286 / 0	-84.3 -84.3	0.19 (1)	I-D	-784 / 0	0.13 (1)	
C-O	-3324 / 0	-84.3 -84.3	0.71 (1)	I-E	0 / 1397	0.35 (1)	
O-P	-3324 / 0	-84.3 -84.3	0.71 (1)	H-E	0 / 305	0.08 (2)	
P-Q	-3324 / 0	-84.3 -84.3	0.71 (1)	K-L	0 / 408	0.00 (1)	
Q-D	-3324 / 0	-84.3 -84.3	0.71 (1)	M-N	0 / 308	0.00 (1)	
D-R	-3324 / 0	-84.3 -84.3	0.71 (1)				
R-S	-3324 / 0	-84.3 -84.3	0.71 (1)				
S-E	-3324 / 0	-84.3 -84.3	0.71 (1)				
E-T	-2303 / 0	-84.3 -84.3	0.23 (1)				
T-N	-2303 / 0	-84.3 -84.3	0.23 (1)				
N-F	-2570 / 0	-84.3 -84.3	0.24 (1)				
F-G	0 / 20	-84.3 -84.3	0.12 (1)				
B-K	0 / 1980	-28.0 -28.0	0.38 (1)				
K-U	0 / 1980	-28.0 -28.0	0.38 (1)				
U-J	0 / 1980	-28.0 -28.0	0.38 (1)				
J-V	0 / 2002	-28.0 -28.0	0.42 (1)				
V-W	0 / 2002	-28.0 -28.0	0.42 (1)				
W-I	0 / 2002	-28.0 -28.0	0.42 (1)				
I-X	0 / 2014	-28.0 -28.0	0.43 (1)				
X-Y	0 / 2014	-28.0 -28.0	0.43 (1)				
Y-H	0 / 2014	-28.0 -28.0	0.43 (1)				
H-Z	0 / 2000	-28.0 -28.0	0.35 (1)				
Z-M	0 / 2000	-28.0 -28.0	0.35 (1)				
M-F	0 / 2000	-28.0 -28.0	0.35 (1)				

FACTORED CONCENTRATED LOADS (LBS)						
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.
JT	3-0-12	-71	-76	---	FRONT	VERT.
O	3-0-12	-145	-145	---	FRONT	VERT.
P	5-0-12	-118	-116	---	FRONT	VERT.
Q	7-0-12	-116	-116	---	FRONT	VERT.
R	9-0-12	-116	-116	---	FRONT	VERT.
S	11-0-12	-118	-118	---	FRONT	VERT.
T	13-0-12	-116	-116	---	FRONT	VERT.
U	1-0-12	-71	-76	---	FRONT	VERT.
V	5-0-12	-71	-76	---	FRONT	VERT.
W	7-0-12	-71	-76	---	FRONT	VERT.

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 25.6 PSF
 DL = 3.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $\frac{1}{360} (0.49")$
 CALCULATED VERT. DEFL.(LL) = $\frac{1}{999} (0.11")$
 ALLOWABLE DEFL.(TL) = $\frac{1}{360} (0.49")$
 CALCULATED VERT. DEFL.(TL) = $\frac{1}{976} (0.18")$

CSI: TC=0.71 (D-E-1), BC=0.43 (H-I-1), WB=0.35 (C-I-1), SS=0.38 (C-D-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 622 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (E) (INPUT = 0.90)
 JSI METAL= 0.54 (E) (INPUT = 1.00)

DRWG NO. TAM 44933-17
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME 285745	TRUSS NAME T80	QUANTITY 1	PLY 1	JOB DESC. 43954 TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 05 11:24:17 2017 Page 2
ID: yh8WV9x7fRuUdYExBfqHxyvgInM-Vug4uOpMNBm1HnxURQxCzk9 uiRDnJO2hpH2n2ygipS

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
X	9-0-12	-71	-76	—	FRONT	VERT	TOTAL
Y	11-0-12	-71	-76	—	FRONT	VERT	TOTAL
Z	13-0-12	-71	-76	—	FRONT	VERT	TOTAL



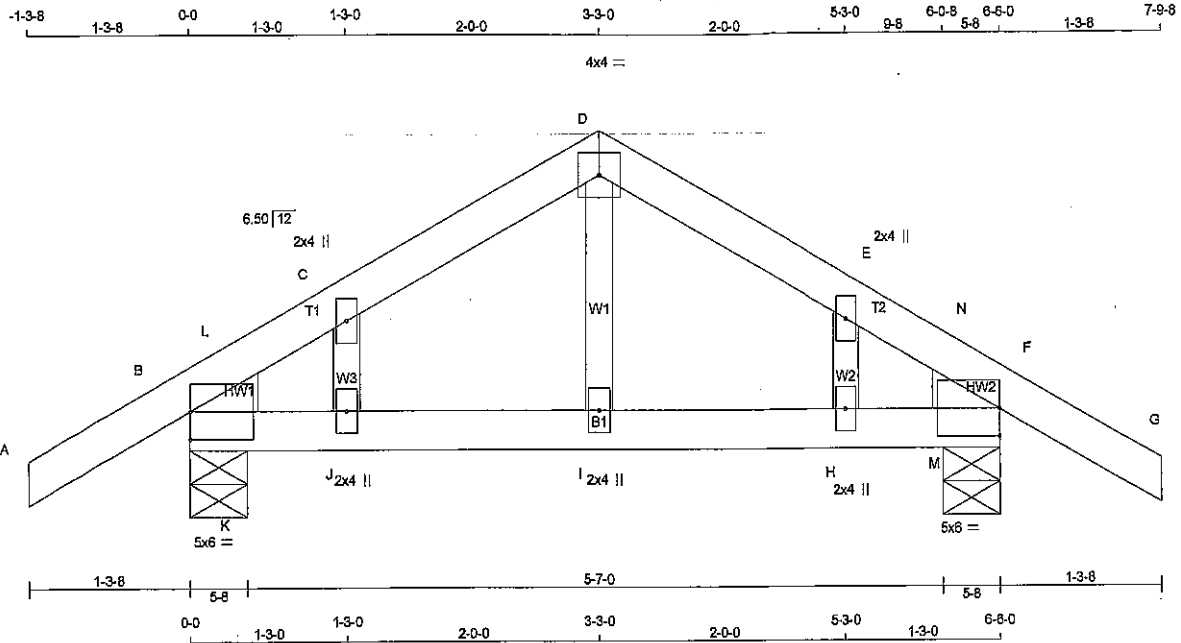
DWG NO. TAM 44933-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285746	G91	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 8 2018 MITek Industries, Inc. Thu Apr 19 13:37:12 2018 Page 1

ID:sGqP1VkJ17Mm8E9Sg?w1yg0Fn-xTz9_3pxa556rdOOK8BS4cpU91kzz2qCy0xVehzP94r



Scale = 1:17.3

TOTAL WEIGHT = 23 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
B - F	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY; SEASONED LUMBER.			

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMBH1-I	MT20	5.0	6.0 2.50
C	TMW+w	MT20	2.0	4.0
D	TTW-p	MT20	4.0	4.0
E	TMW+w	MT20	2.0	4.0
F	TMBH1-I	MT20	5.0	6.0 2.50
H, I, J				
H	BMW+w	MT20	2.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	VERT	HQRZ	GROSS REACTION	DOWN	GROSS REACTION	BRG	HEEL	IN-SX	WEDGE
B	480	0	480	0	0	5-8	5-8	2x4 L	
F	480	0	480	0	0	5-8	5-8	2x4 R	

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	377	238 / 0	68 / 0	0 / 0	0 / 0	73 / 0	0 / 0
F	377	238 / 0	68 / 0	0 / 0	0 / 0	73 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	MAX. FORCE (LBS)	VERT. LOAD LC1	MAX. (PLF)	MAX. UNBRAC LENGTH	MEMB.	MAX. FORCE (LBS)	MAX. (LC)
FR-TO		FROM	TO		FR-TO		
A-B	0 / 17	-84.3	-84.3 0.11 (1)	10.00	I-D	0 / 151	0.03 (2)
B-L	-525 / 0	-84.3	-84.3 0.11 (1)	6.25	H-E	-81 / 26	0.01 (1)
L-C	-322 / 0	-84.3	-84.3 0.05 (1)	6.25	J-C	-81 / 26	0.01 (1)
C-D	-298 / 0	-84.3	-84.3 0.06 (1)	6.25	K-L	0 / 225	0.00 (1)
D-E	-298 / 0	-84.3	-84.3 0.06 (1)	6.25	M-N	0 / 227	0.00 (1)
E-N	-322 / 0	-84.3	-84.3 0.05 (1)	6.25			
N-F	-527 / 0	-84.3	-84.3 0.11 (1)	6.25			
F-G	0 / 17	-84.3	-84.3 0.11 (1)	10.00			
B-K	0 / 267	-28.0	-28.0 0.11 (1)	10.00			
K-J	0 / 267	-28.0	-28.0 0.11 (1)	10.00			
J-I	0 / 261	-28.0	-28.0 0.08 (1)	10.00			
I-H	0 / 261	-28.0	-28.0 0.08 (1)	10.00			
H-M	0 / 267	-28.0	-28.0 0.11 (1)	10.00			
M-F	0 / 267	-28.0	-28.0 0.11 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.22")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.22")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.01")

CSI: TC=0.11/1.00 (F-N:1), BC=0.11/1.00 (H-M:1),
WB=0.03/1.00 (D-I:2), SSI=0.10/1.00 (F-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

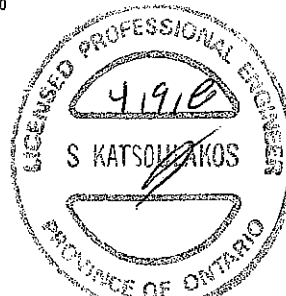
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PS) (PL) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1887 822 2264 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.66 (F) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)



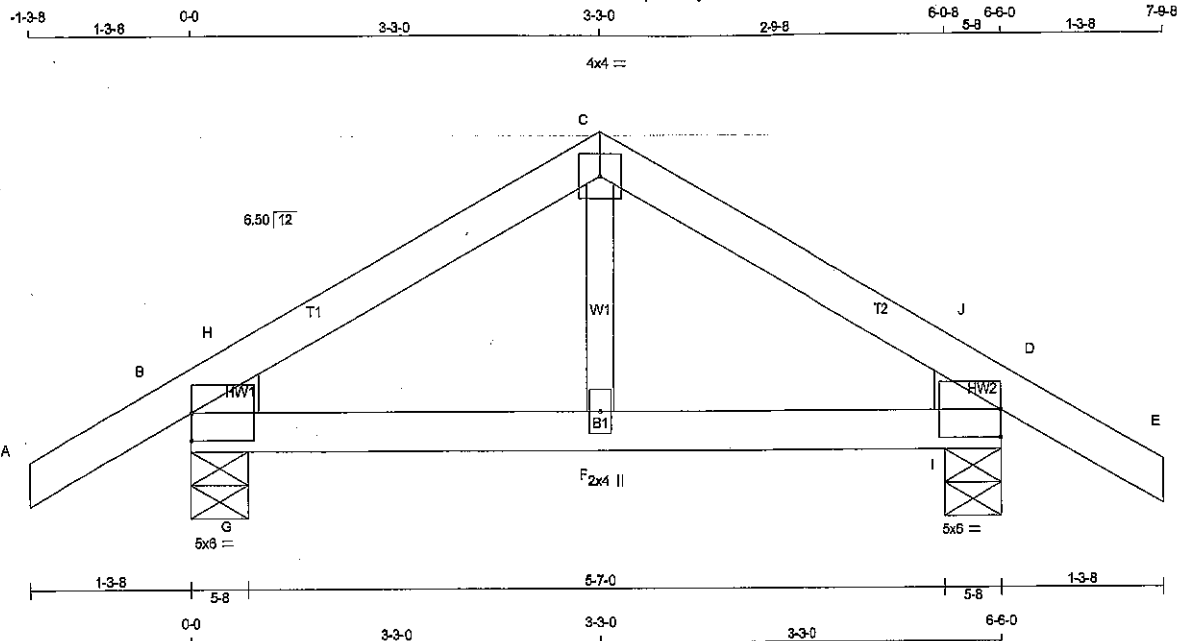
DRWG NO. TAM 21215-08
STRUCTURAL
COMPONENT ONLY

JOB NAME 285746	TRUSS NAME T91	QUANTITY 3	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Thu Apr 19 13:37:14 2018 Page 1

ID:sGqP1Vv7j17Mm8E9Sg?w1yg0Fn-ur5wPlqB6iLq5wYnRZDw91uqgqQWRYGVQKQdzP94p



TOTAL WEIGHT = 3 X 22 = 67 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
		DRY; SEASONED LUMBER.		

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-I	MT20	5.0	6.0	2.50	
C	TTW-p	MT20	4.0	4.0		
D	TMBH1-I	MT20	5.0	6.0	2.50	
F	BMW-w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX	HEEL WEDGE
	VERT	HORZ	DOWN	HORZ			
B	480	0	480	0	5-8	5-8	2x4 L
D	480	0	480	0	5-8	5-8	2x4 R

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	377	236 / 0	68 / 0	0 / 0	0 / 0	73 / 0	0 / 0
D	377	236 / 0	68 / 0	0 / 0	0 / 0	73 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 17	-84.3	-84.3	0.11 (1)	10.00	F-C	0 / 165
B-H	-411 / 0	-84.3	-84.3	0.05 (1)	6.25	G-H	-34 / 113
H-C	-355 / 0	-84.3	-84.3	0.08 (1)	6.25	I-J	-33 / 114
C-J	-355 / 0	-84.3	-84.3	0.08 (1)	6.25		
J-D	-412 / 0	-84.3	-84.3	0.05 (1)	6.25		
D-E	0 / 17	-84.3	-84.3	0.11 (1)	10.00		
B-G	0 / 305	-28.0	-28.0	0.06 (1)	10.00		
G-F	0 / 305	-28.0	-28.0	0.10 (1)	10.00		
F-I	0 / 305	-28.0	-28.0	0.10 (1)	10.00		
I-D	0 / 305	-28.0	-28.0	0.08 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.22")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.22")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.01")

CSI: TC=0.11/1.00 (A-B:1), BC=0.10/1.00 (F-G:1), WB=0.04/1.00 (C-F:2), SS=0.09/1.00 (C-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1687 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (D) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

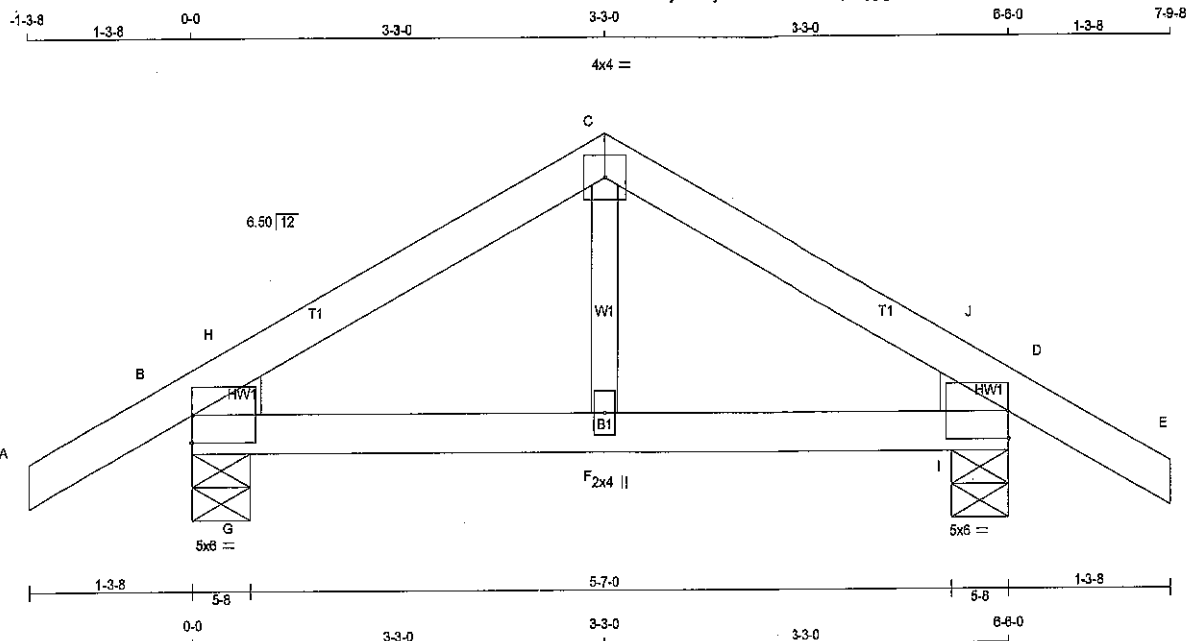


DRWG NO. TAM 2429-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
285745	T93	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID: yh8WY9x7fRuUdYExBfqHxygInM-RHorJ4qduDclW54fYrzg39EUWCoFIL87m9xygipQ



Scale = 1:17.3

TOTAL WEIGHT = 2 X 22 = 44 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4 DRY	No.2	SPF
C - E	2x4 DRY	No.2	SPF
B - D	2x4 DRY	No.2	SPF
ALL WEBS	2x3 DRY	No.2	SPF
DRY: SEASONED LUMBER.			

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMBH1-I	MT20	5.0	6.0
C	TTW-p	MT20	4.0	4.0
D	TMBH1-I	MT20	5.0	6.0
F	BMW-w	MT20	2.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG	HEEL WEDGE
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX
B	480	0	480	0	0	5-8
D	480	0	480	0	0	5-8

UNFACTORED REACTIONS

JT	COMBINED	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	377	236 / 0	68 / 0	0 / 0	0 / 0	0 / 0	73 / 0	0 / 0	0 / 0
D	377	236 / 0	68 / 0	0 / 0	0 / 0	0 / 0	73 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. LOAD (LC1)	MEMB.	FORCE (LBS)
FR-TO				FR-TO	
A-B	0 / 17	-84.3	-84.3 0.11 (1)	F-C	0 / 165
B-H	-412 / 0	-84.3	-84.3 0.05 (1)	G-H	-34 / 113
H-C	-355 / 0	-84.3	-84.3 0.08 (1)	I-J	-34 / 113
C-J	-355 / 0	-84.3	-84.3 0.08 (1)		
J-D	-412 / 0	-84.3	-84.3 0.05 (1)		
D-E	0 / 17	-84.3	-84.3 0.11 (1)		
B-G	0 / 305	-28.0	-28.0 0.08 (1)		
G-F	0 / 305	-28.0	-28.0 0.10 (1)		
F-I	0 / 305	-28.0	-28.0 0.10 (1)		
I-D	0 / 305	-28.0	-28.0 0.08 (1)		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, BCSC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.22")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.22")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.11 (A-B:1), BC=0.10 (F-G:1), WB=0.04 (C-F:2), SSI=0.09 (C-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1067 622 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (D) (INPUT = 0.90)
JSI METAL= 0.10 (D) (INPUT = 1.00)



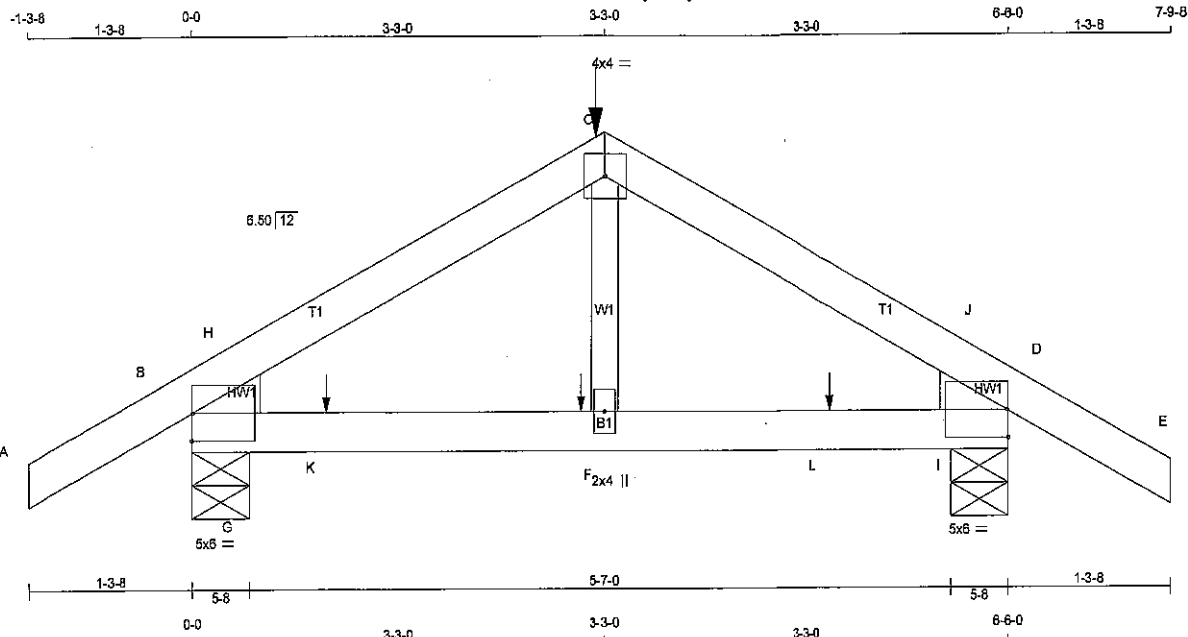
DWG NO. TAM 44935-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43854	DRWG NO.
285745	T93Z	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 Mitek Industries, Inc. Tue Sep 05 11:24:20 2017 Page 1

ID: yh8WY9x7fRuUdYExBfqHxyglnM-vTMDWQrFfVkc8Ff36YUvbNnfKwYt_kyUNnWjONygiP



Scale = 1:17.3

TOTAL WEIGHT = 22 lb

<u>LUMBER</u>					
N. L. G. A. RULES					
CHORDS		SIZE	LUMBER		DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
B - D	2x4	DRY	No.2	SPF	
ALL WEBS		2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.					

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-I	MT20	5.0	6.0		
C	TTW-p	MT20	4.0	4.0		
D	TMBH1-I	MT20	5.0	6.0		
F	BMW-w	MT20	2.0	4.0		

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 2.4 lbs FACTORED DOWN AT 3-3-0 ON TOP CHORD, AND AT 1-0-12, AND AT 3-0-12, AND AT 5-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

BEARINGS								
	FACTORED		MAXIMUM FACTORED			INPUT	REQRD	
	GROSS REACTION		GROSS REACTION			BRG	BRG	HEEL
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	WEDGE
B	481	0	481	0	0	5-8	5-8	2x4 L
D	481	0	481	0	0	5-8	5-8	2x4 R

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	378	236 / 0	66 / 0	0 / 0	0 / 0	74 / 0	0 / 0
D	378	236 / 0	66 / 0	0 / 0	0 / 0	74 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	MEMB.
FR-TO		FROM TO		FR-TO			
A-B	0 / 17	-84.3 -84.3 0.12 (1)	10.00	F-C	0 / 165	0.04 (2)	
B-H	-414 / 0	-84.3 -84.3 0.07 (1)	8.25	G-H	-33 / 114	0.00 (1)	
H-C	-357 / 0	-84.3 -84.3 0.10 (1)	6.25	I-J	-33 / 114	0.00 (1)	
C-J	-357 / 0	-84.3 -84.3 0.10 (1)	6.25				
J-D	-414 / 0	-84.3 -84.3 0.07 (1)	10.00				
D-E	0 / 17	-84.3 -84.3 0.12 (1)	10.00				
B-G	0 / 307	-28.0 -28.0 0.09 (1)	10.00				
G-K	0 / 307	-28.0 -28.0 0.11 (1)	10.00				
K-F	0 / 307	-28.0 -28.0 0.11 (1)	10.00				
F-L	0 / 307	-28.0 -28.0 0.11 (1)	10.00				
L-I	0 / 307	-28.0 -28.0 0.11 (1)	10.00				
I-D	0 / 307	-28.0 -28.0 0.09 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	D/R.	TYPE
C	3-3-0	-2	-	-	FRONT	VERT	TOTAL
F	3-0-12	-	-	-	FRONT	VERT	TOTAL
K	1-0-12	-	-	-	FRONT	VERT	TOTAL
L	5-0-12	-	-	-	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 25.6	PSF
	DL	= 3.0	PSF
BOT CH.	LL	= 10.5	PSF
	DL	= 7.0	PSF
TOTAL LOAD = 46.1 PSF			

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.22")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.22")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.12 (A-B:1), BC=0.11 (F-G:1), WB=0.04 (C-F:2), SSI=0.10 (C-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

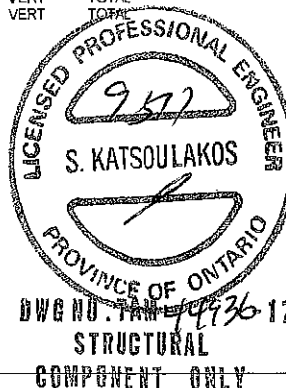
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	618	354	1687
	622	2264	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (D) (INPUT = 0.90)
JSI METAL= 0.10 (D) (INPUT = 1.00)

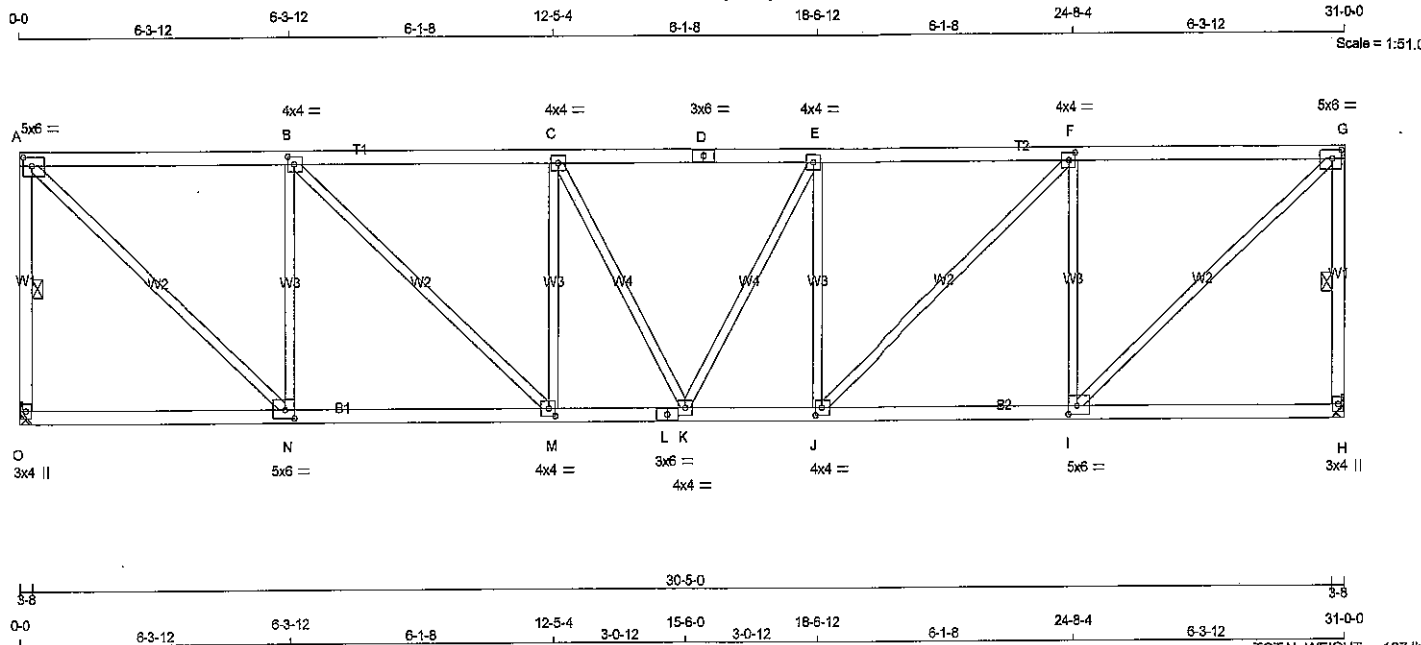


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
285745	T145	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:yh8WY9x7fRuUdYExBfqHxyglnM-vTMDWQrFfWkc8Ff36YUvbNnWvwSH_agUNnWjONygiP



TOTAL WEIGHT = 137 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
O - A	2x4	DRY No.2	SPF
A - D	2x4	DRY No.2	SPF
D - G	2x4	DRY No.2	SPF
H - G	2x4	DRY No.2	SPF
O - L	2x4	DRY No.2	SPF
L - H	2x4	DRY No.2	SPF
ALL WEBS 2x3 DRY No.2 SPF			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
A	TMWW-t	MT20	5.0	6.0 2.25 2.50
B	TMWW-t	MT20	4.0	4.0 2.00 1.75
C	TMWW-t	MT20	4.0	4.0
D	TS-t	MT20	3.0	6.0
E	TMWW-t	MT20	4.0	4.0
F	TMWW-t	MT20	4.0	4.0 2.00 1.75
G	TMWW-t	MT20	5.0	6.0 2.25 2.50
H	BMV1-p	MT20	3.0	4.0
I	BMWW-t	MT20	5.0	6.0 2.25 2.50
J	BMWW-t	MT20	4.0	4.0 2.00 1.75
K	BMWW-t	MT20	4.0	4.0
L	BS-t	MT20	3.0	6.0
M	BMWW-t	MT20	4.0	4.0 2.00 1.75
N	BMWW-t	MT20	5.0	6.0 2.25 2.50
O	BMV1-p	MT20	3.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
O	1740	0	1740	0	0	HANGER BY OTHERS
H	1740	0	1740	0	0	MIN. SEAT SIZE: 3-8 HANGER BY OTHERS MIN. SEAT SIZE: 3-8

UNFACTORED REACTIONS

JT	1ST LCASE MAX./MIN. COMPONENT REACTIONS						
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1429	793 / 0	328 / 0	0 / 0	0 / 0	310 / 0	0 / 0
H	1429	793 / 0	328 / 0	0 / 0	0 / 0	310 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.81 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF A-O, G-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 MAX. (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
O-A	-1669 / 0	0.0	0.0 0.29 (1)	I-G	0 / 2175	0.49 (1)	
A-B	-1615 / 0	-84.3	-84.3 0.58 (1)	A-N	0 / 2175	0.49 (1)	
B-C	-2326 / 0	-84.3	-84.3 0.68 (1)	I-F	-1254 / 0	0.70 (1)	
C-D	-2339 / 0	-84.3	-84.3 0.50 (1)	N-B	-1254 / 0	0.70 (1)	
D-E	-2339 / 0	-84.3	-84.3 0.50 (1)	J-F	0 / 971	0.22 (1)	
E-F	-2326 / 0	-84.3	-84.3 0.68 (1)	B-M	0 / 971	0.22 (1)	
F-G	-1615 / 0	-84.3	-84.3 0.58 (1)	J-E	-524 / 0	0.29 (1)	
H-G	-1669 / 0	0.0	0.0 0.29 (1)	M-C	-524 / 0	0.29 (1)	
O-N	0 / 0	-28.0	-28.0 0.28 (3)	C-K	0 / 51	0.01 (3)	
N-M	0 / 1815	-28.0	-28.0 0.47 (2)	K-E	0 / 51	0.01 (3)	
M-L	0 / 2326	-28.0	-28.0 0.44 (1)				
L-K	0 / 2326	-28.0	-28.0 0.44 (1)				
K-J	0 / 2326	-28.0	-28.0 0.44 (1)				
J-I	0 / 1815	-28.0	-28.0 0.47 (2)				
I-H	0 / 0	-28.0	-28.0 0.28 (3)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 25.8	PSF
	DL	= 3.0	PSF
BOT CH.	LL	= 10.5	PSF
	DL	= 7.0	PSF
TOTAL LOAD		= 46.1	PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 089-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/999 (0.24")

CSI: TC=0.66 (E-F:1), BC=0.47 (M-N:2), WB=0.70 (F-I:1), SSI=0.25 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

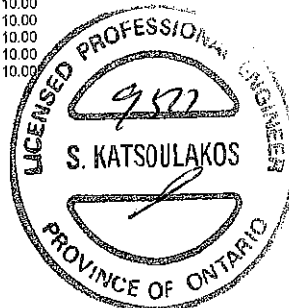
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1657 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (F) (INPUT = 0.90)

JSI METAL= 0.81 (L) (INPUT = 1.00)



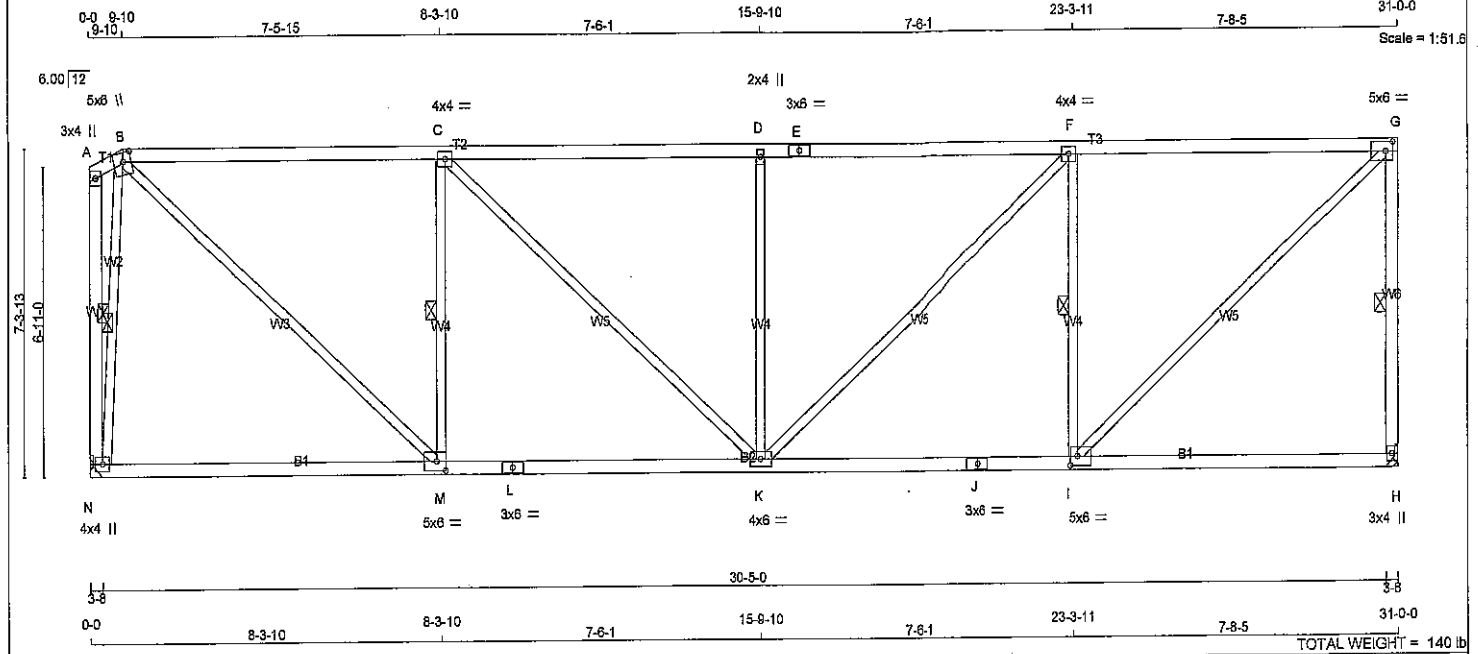
DRWG NO. TAM 4493717
STRUCTURAL
COMPONENT ONLY

JOB NAME 285745	TRUSS NAME T146	QUANTITY 1	PLY 1	JOB DESC. 43954	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID: yh8WY9x7fRuUdyExBfqHxyglnM-OgwbkmsTQsTmPEFgG?68aJcHKlJ3BecRFgWpygipO



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	DESCR.
A - B	2x4	DRY	No.2
B - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
H - G	2x4	DRY	No.2
N - A	2x4	DRY	No.2
N - L	2x4	DRY	No.2
L - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2

ALL WEBS EXCEPT			
SIZE	DESCR.	SPF	
2x3	DRY	No.2	SPF

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMV+p	MT20	3.0	4.0	
B	TTWW+m	MT20	5.0	6.0	2.50 2.25
C	TMWW-t	MT20	4.0	4.0	
D	TMWW+w	MT20	2.0	4.0	
E	TS-t	MT20	3.0	8.0	
F	TMWW-t	MT20	4.0	4.0	
G	TMWW-t	MT20	5.0	8.0	2.50 2.00
H	BMV1+p	MT20	3.0	4.0	
I	BMWW-t	MT20	5.0	6.0	2.50 2.00
J	BS-t	MT20	3.0	6.0	
K	BMWW-t	MT20	4.0	6.0	
L	BS-t	MT20	3.0	6.0	
M	BMWW-t	MT20	5.0	6.0	2.50 2.50
N	BMV1+p	MT20	4.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS					
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG	
H	1740 0	1740 0	0 0	0 0	
N	1740 0	1740 0	0 0	0 0	

UNFACTORED REACTIONS							
JT	1ST CASE	MAX./MIN. COMPONENT REACTIONS					
H	1429	793 / 0	326 / 0	0 / 0	0 / 0	310 / 0	0 / 0
N	1429	793 / 0	326 / 0	0 / 0	0 / 0	310 / 0	0 / 0

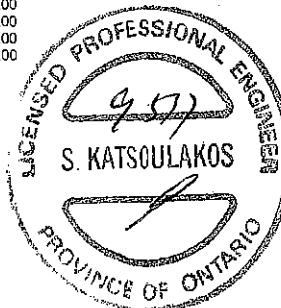
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.47 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-H, C-M, A-N, B-N, F-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING					
TOTAL LOAD CASES: (4)					
CHORDS					
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC2)	MAX. FACTORED VERT. LOAD (LC3)
FR-TO					
A-B	0 / 0	-84.3	-84.3	0.01 (1)	10.00
B-C	-1603 / 0	-84.3	-84.3	0.89 (1)	3.94
C-D	-1978 / 0	-84.3	-84.3	0.97 (1)	3.55
D-E	-1978 / 0	-84.3	-84.3	1.00 (1)	3.47
E-F	-1978 / 0	-84.3	-84.3	1.00 (1)	3.47
F-G	-1530 / 0	-84.3	-84.3	0.92 (1)	3.96
G-H	-1654 / 0	0.0	0.0	0.40 (1)	5.16
H-A	-34 / 0	0.0	0.0	0.01 (1)	6.25
N-M	0 / 184	-28.0	-28.0	0.48 (2)	10.00
M-L	0 / 1803	-28.0	-28.0	0.85 (2)	10.00
L-K	0 / 1803	-28.0	-28.0	0.85 (2)	10.00
K-J	0 / 1530	-28.0	-28.0	0.59 (2)	10.00
J-I	0 / 1530	-28.0	-28.0	0.59 (2)	10.00
I-H	0 / 0	-28.0	-28.0	0.41 (3)	10.00



DWG NO. TAM 44938-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA		
SPECIFIED LOADS:		
TOP CH. LL	=	25.6 PSF
DL	=	3.0 PSF
BOT CH. LL	=	10.5 PSF
DL	=	7.0 PSF
TOTAL LOAD	=	46.1 PSF
SPACING	=	24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (1.03")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.19")
ALLOWABLE DEFL. (TL) = $L/360$ (1.03")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.32")

CSI: TC=1.00 (D-F:1), BC=0.65 (K-M:2), WB=0.55 (D-K:1), SSI=0.31 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

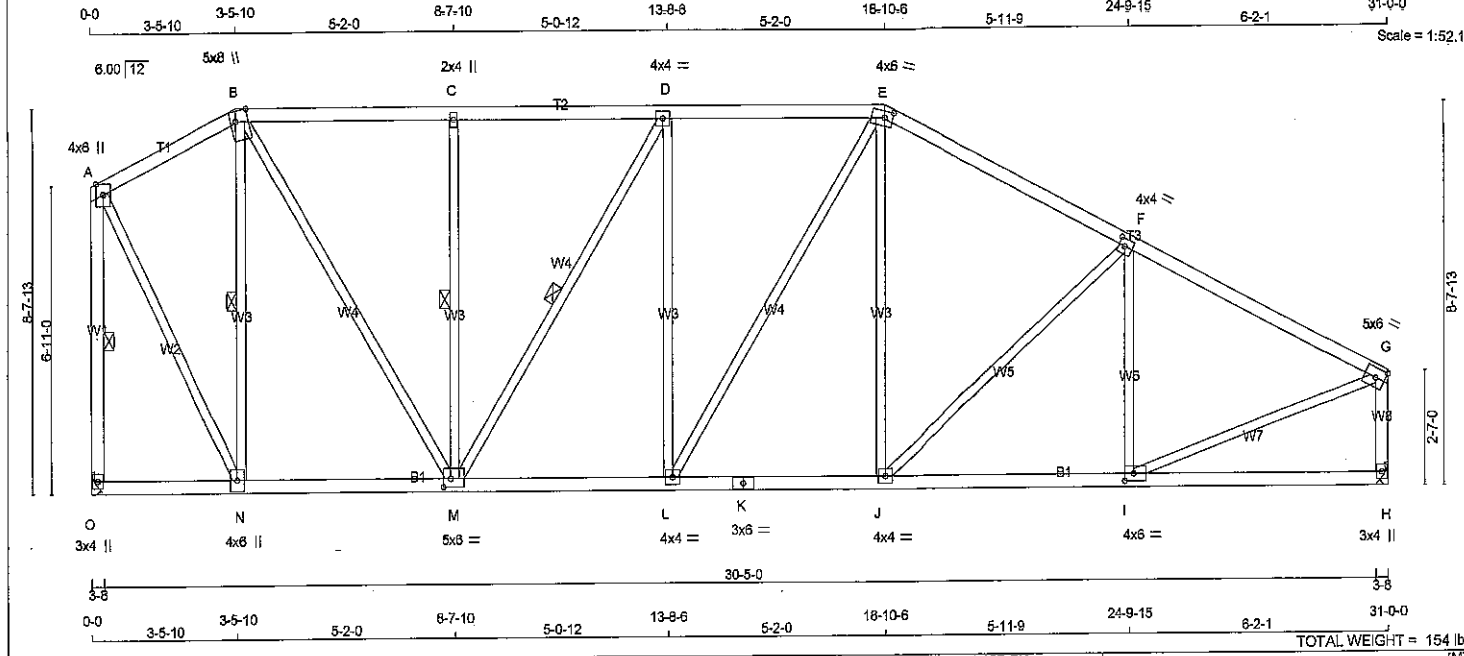
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)	MAX MIN	MAX MIN	MAX MIN
MT20	518	354	1667	822	2284

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.46 (L) (INPUT = 1.00)



LUMBER

N. L. G. A. RULES

CHORDS SIZE

LUMBER

DESCR.

A - B

2x4

DRY

No.2

SPF

B - E

2x4

DRY

No.2

SPF

E - G

2x4

DRY

No.2

SPF

O - A

2x4

DRY

No.2

SPF

H - G

2x4

DRY

No.2

SPF

O - K

2x4

DRY

No.2

SPF

K - H

2x4

DRY

No.2

SPF

ALL WEBS

2x3

DRY

No.2

SPF

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE

PLATES

W

LEN

Y

X

A

TMVW+p

MT20

4.0

6.0

Edge

B

TTWW+m

MT20

5.0

8.0

Edge

3.50

C

TMW+w

MT20

2.0

4.0

D

TMWW-t

MT20

4.0

4.0

E

TTWW-m

MT20

4.0

6.0

1.75

2.25

F

TMWW-t

MT20

4.0

4.0

2.00

1.75

G

TMVW-t

MT20

5.0

6.0

Edge

H

BMV1+p

MT20

3.0

4.0

I

BMWW-t

MT20

4.0

6.0

2.00

2.50

J

BMWW-t

MT20

4.0

4.0

K

BS-t

MT20

3.0

6.0

L

BMWW-t

MT20

4.0

4.0

M

BMWWW-t

MT20

5.0

6.0

2.25

2.00

N

BMWW+t

MT20

4.0

6.0

O

BMV1+p

MT20

3.0

4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED GROSS REACTION

MAXIMUM FACTORED GROSS REACTION

INPUT BRG

REQD BRG

JT

VERT

HORZ

DOWN

HORZ

UPLIFT

IN-SX

IN-SX

O

1740

0

1740

0

0

H

1740

0

1740

0

0

HANGER BY OTHERS

MIN. SEAT SIZE: 3-8

HANGER BY OTHERS

MIN. SEAT SIZE: 3-8

UNFACTORED REACTIONS

1ST LCASE

MAX./MIN. COMPONENT REACTIONS

JT

COMBINED

SNOW

LIVE

PERM.LIVE

WIND

DEAD

SOIL

O

1429

793 / 0

326 / 0

0 / 0

0 / 0

310 / 0

0 / 0

H

1429

793 / 0

326 / 0

0 / 0

0 / 0

310 / 0

0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.12 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF B-N, C-M, D-M, A-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

WEBS

MEMB.

MAX. FACTORED FORCE (LBS)

FACTORED VERT. LOAD (PLF)

MAX. CS1 (LC)

MAX. UNBRACED LENGTH

MEMB.

MAX. FACTORED FORCE (LBS)

MAX. CS1 (LC)

FR-TO

FROM

TO

FR-TO

A-B

-742 / 0

-84.3

-84.3

0.18 (1)

6.25

N-B

-1088 / 0

0.52 (1)

B-C

-1328 / 0

-84.3

-84.3

0.33 (1)

5.22

B-M

0 / 1289

0.29 (1)

C-D

-1328 / 0

-84.3

-84.3

0.34 (1)

5.20

M-C

-468 / 0

0.22 (1)

D-E

-1624 / 0

-84.3

-84.3

0.36 (1)

4.79

M-D

-572 / 0

0.39 (1)

E-F

-1755 / 0

-84.3

-84.3

0.66 (1)

4.24

L-D

0 / 203

0.05 (3)

F-G

-1866 / 0

-84.3

-84.3

0.69 (1)

4.12

L-E

0 / 145

0.03 (1)

O-A

-1706 / 0

0.0

0.0

0.38 (1)

5.08

J-E

0 / 351

0.08 (2)

H-G

-1670 / 0

0.0

0.0

0.22 (1)

6.42

J-F

-169 / 0

0.27 (1)

I-F

-430 / 31

0.21 (1)

O-N

0 / 0

-28.0

-28.0

0.13 (2)

10.00

A-N

0 / 1394

0.31 (1)

N-M

0 / 650

-28.0

-28.0

0.21 (2)

10.00

I-G

0 / 1604

0.41 (1)

M-L

0 / 1624

-28.0

-28.0

0.33 (1)

10.00

L-K

0 / 1547

-28.0

-28.0

0.34 (2)

10.00

K-J

0 / 1547

-28.0

-28.0

0.34 (2)

10.00

J-I

0 / 1693

-28.0

-28.0

0.46 (2)

10.00

I-H

0 / 0

-28.0

-28.0

0.26 (3)

10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP

CH.

LL

=

25.6

PSF

DL

=

3.0

PSF

BOT

CH.

LL

=

10.5

PSF

DL

=

7.0

PSF

TOTAL LOAD

=

46.1

PSF

SPACING

=

24.0

IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014

- CSA 088-09

- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")

CALCULATED VERT. DEFL.(LL)= L/999 (0.09")

ALLOWABLE DEFL.(TL)= L/360 (1.03")

CALCULATED VERT. DEFL.(TL)= L/999 (0.15")

CSI: TC=0.69 (F-G:1), BC=0.48 (I-J:2), WB=0.52 (B-N:1), SSI=0.22 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)

JSI METAL= 0.49 (G) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER

9577

S. KATSOULAKOS

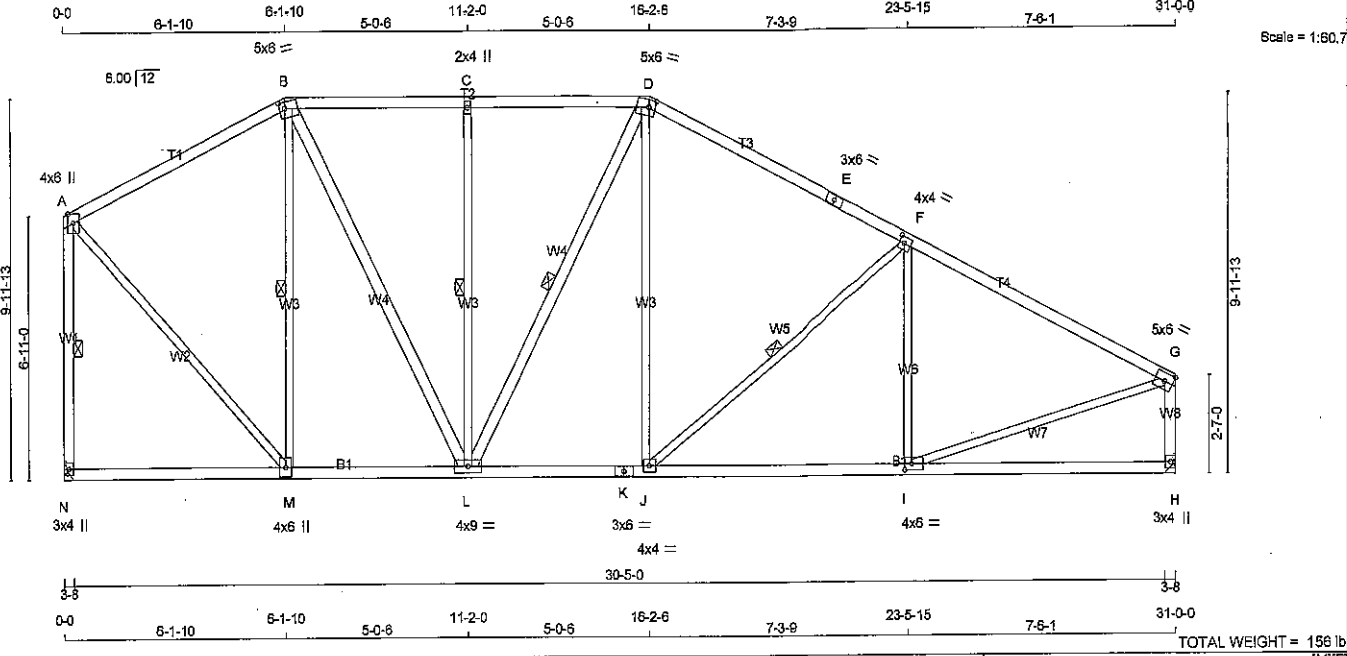
PROVINCE OF ONTARIO

DWG NO. TAM 44939-17

STRUCTURAL

COMPONENT ONLY





LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY No.2	SPF
B - D	2x4	DRY No.2	SPF
D - E	2x4	DRY No.2	SPF
E - G	2x4	DRY No.2	SPF
N - A	2x4	DRY No.2	SPF
H - G	2x4	DRY No.2	SPF
N - K	2x4	DRY No.2	SPF
K - H	2x4	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF
B - L	2x4	DRY No.2	SPF
L - D	2x4	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMW+p	MT20	4.0	6.0	Edge	
B	TTWW-m	MT20	5.0	6.0	2.00	1.75
C	TMW+w	MT20	2.0	4.0		
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0	2.00	1.75
G	TMW-t	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	6.0	2.00	2.25
J	BMWW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMWW-t	MT20	4.0	9.0		
M	BMWW-t	MT20	4.0	6.0		
N	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
N	1740	0	1740	0	0	0
H	1740	0	1740	0	0	0

MIN. SEAT SIZE: 3-8
HANGER BY OTHERS

UNFACTORED REACTIONS

JT	MAX./MIN. COMPONENT REACTIONS						
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1429	793 / 0	326 / 0	0 / 0	0 / 0	310 / 0	0 / 0
H	1429	793 / 0	326 / 0	0 / 0	0 / 0	310 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.93 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF B-M, C-L, D-L, F-J, A-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO				
A-B	-1012 / 0	-84.3 -84.3 0.43 (1)	5.85	0.52 (1)
B-C	-1311 / 0	-84.3 -84.3 0.29 (1)	5.32	0.14 (1)
C-D	-1311 / 0	-84.3 -84.3 0.29 (1)	5.32	0.35 (1)
D-E	-1594 / 0	-84.3 -84.3 0.69 (1)	4.33	0.12 (1)
E-F	-1594 / 0	-84.3 -84.3 0.69 (1)	4.33	0.12 (2)
F-G	-1938 / 0	-84.3 -84.3 0.75 (1)	3.93	0.31 (1)
N-A	-1670 / 0	0.0 0.0 0.37 (1)	5.14	0.19 (1)
H-G	-1656 / 0	0.0 0.0 0.22 (1)	6.44	0.30 (1)
N-M	0 / 0	-28.0 -28.0 0.25 (2)	10.00	
M-L	0 / 896	-28.0 -28.0 0.36 (2)	10.00	
L-K	0 / 1400	-28.0 -28.0 0.36 (2)	10.00	
K-J	0 / 1400	-28.0 -28.0 0.36 (2)	10.00	
J-I	0 / 1762	-28.0 -28.0 0.60 (2)	10.00	
I-H	0 / 0	-28.0 -28.0 0.40 (3)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(L) = L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/999 (0.13")
ALLOWABLE DEFL.(TL) = L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/999 (0.22")

CSI: TC=0.75 (F-G-1), BC=0.60 (I-J-2), WB=0.52 (B-M-1), SSI=0.27 (F-G-1)

OOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

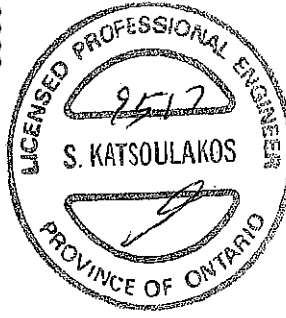
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1668

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.87 (M) (INPUT = 0.80)
JSI METAL= 0.51 (G) (INPUT = 1.00)

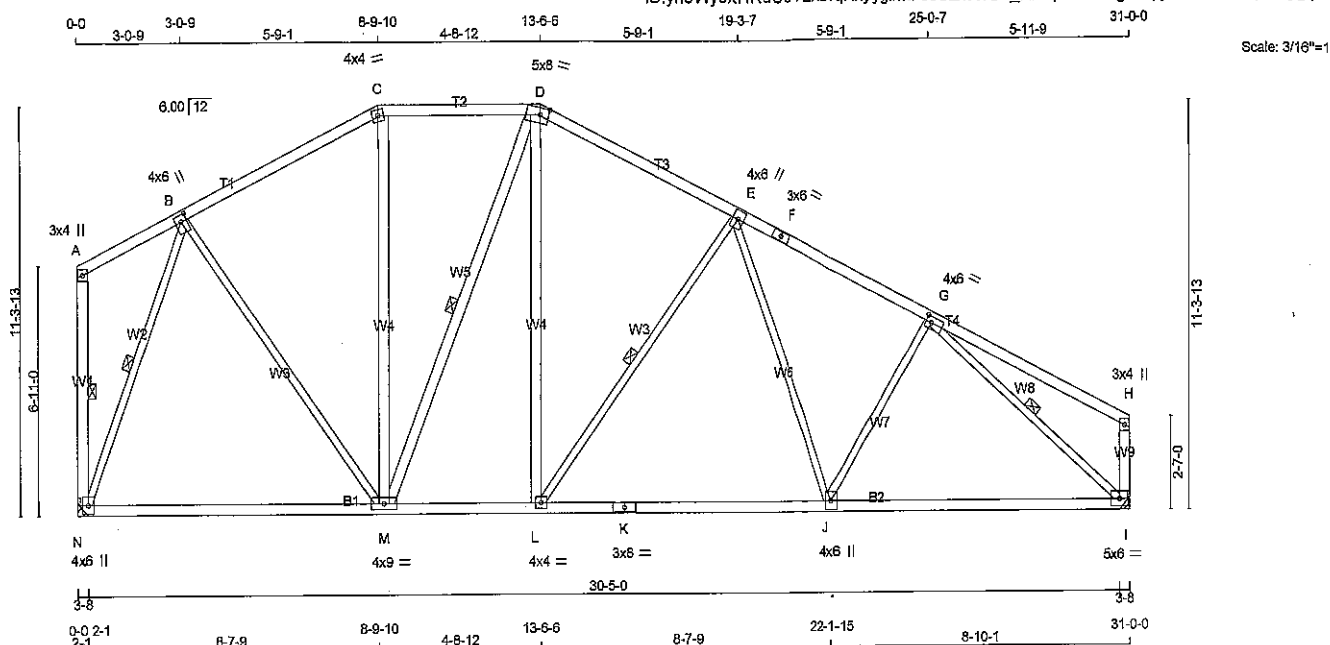


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43854	DRWG NO.
285745	T149	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 162 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - A	2x4	DRY	No.2
A - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
K - I	2x4	DRY	No.2

ALL WEBS EXCEPT			
M - C	2x4	DRY	No.2
M - D	2x4	DRY	No.2
L - D	2x4	DRY	No.2

DRY; SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW+t	MT20	4.0	6.0	2.25	2.00
C	TTW-m	MT20	4.0	4.0		
D	TTWW-m	MT20	5.0	8.0	2.25	3.00
E	TMWW+t	MT20	4.0	6.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	6.0	2.00	2.00
H	TMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	5.0	6.0		
J	BMWW+t	MT20	4.0	6.0		
K	BS-t	MT20	3.0	8.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMWWW-t	MT20	4.0	9.0		
N	BMVW1+p	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT		REQRD	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	BRG	BRG
N	1740	0	1740	0	0				
I	1740	0	1740	0	0				

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1429	793 / 0	326 / 0	0 / 0	0 / 0	310 / 0	0 / 0
I	1429	793 / 0	326 / 0	0 / 0	0 / 0	310 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF B-N, D-M, E-L, A-N, G-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	CS1 (LC)	MEMB.	FORCE (LBS)	MAX. CS1 (LC)	UNBRACED LENGTH
FR-TO		FROM	TO	FR-TO			
A-B	0 / 37	-84.3	-84.3	0.34 (1)	10.00	N-B	-1698 / 0
B-C	-1159 / 0	-84.3	-84.3	0.35 (1)	5.50	B-M	0 / 722
C-D	-1020 / 0	-84.3	-84.3	0.25 (1)	5.90	M-C	0 / 231
D-E	-1367 / 0	-84.3	-84.3	0.36 (1)	5.16	M-D	-498 / 0
E-F	-1861 / 0	-84.3	-84.3	0.36 (1)	4.58	L-D	0 / 889
F-G	-1861 / 0	-84.3	-84.3	0.36 (1)	4.58	L-E	-656 / 0
G-H	0 / 21	-84.3	-84.3	0.36 (1)	10.00	E-J	0 / 350
H-A	-48 / 0	0.0	0.0	0.01 (1)	6.25	J-G	0 / 227
I-H	-201 / 0	0.0	0.0	0.03 (1)	7.81	G-I	-2171 / 0
N-M	0 / 594	-28.0	-28.0	0.53 (2)	10.00		
M-L	0 / 1212	-28.0	-28.0	0.61 (2)	10.00		
L-K	0 / 1589	-28.0	-28.0	0.77 (2)	10.00		
K-J	0 / 1589	-28.0	-28.0	0.77 (2)	10.00		
J-I	0 / 1624	-28.0	-28.0	0.77 (2)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL)= L/999 (0.26")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL)= L/861 (0.43")

CS1: TC=0.38 (G-H:1), BC=0.77 (I-J:2), WB=0.88 (G-I:1), SS1=0.21 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

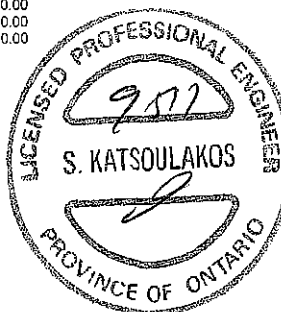
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 622 2284 1956

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (G) (INPUT = 0.90)
JSI METAL= 0.65 (K) (INPUT = 1.00)



DWG NO. TAM 4494/17

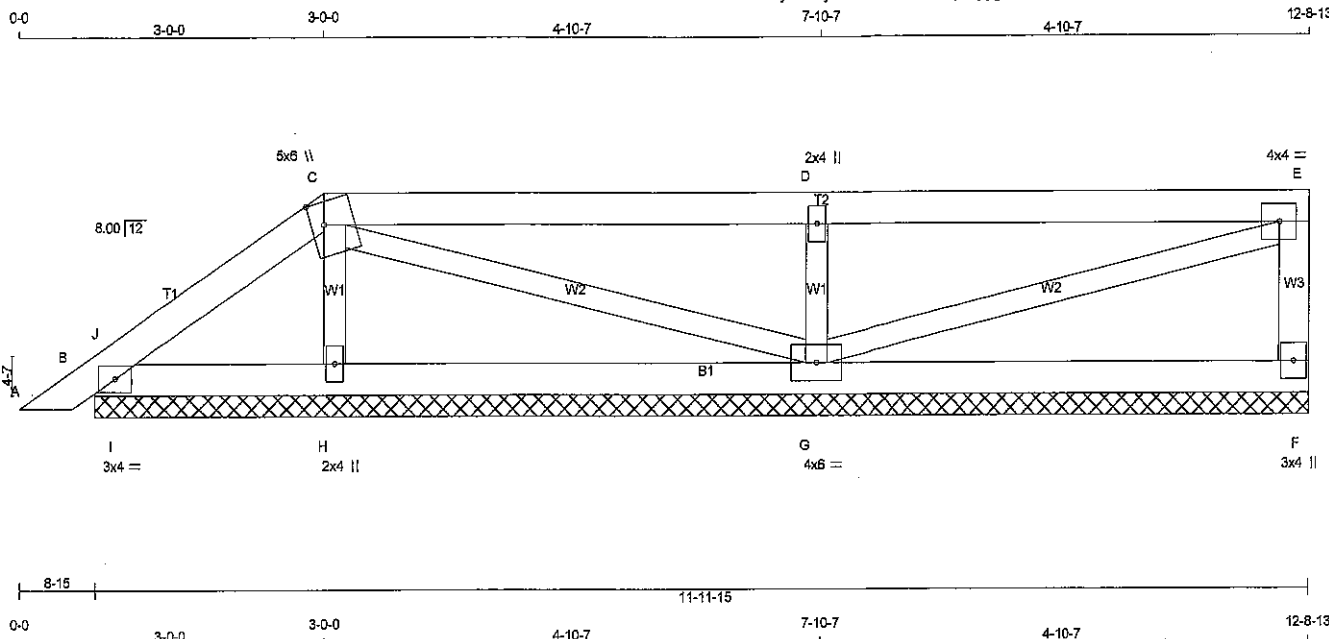
STRUCTURAL

COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
285745	P6	5	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1:21.5

LUMBER				DESCR.	
N. L. G. A. RULES				SPF	
CHORDS	SIZE	LUMBER	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
E - F	2x4	DRY	No.2	SPF	
B - F	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED		MAXIMUM FACTORED		INPUT	REQD
	GROSS REACTION		GROSS REACTION		BRG	BRG
F	210	0	210	0	11-11-15	11-11-15
B	194	0	194	0	11-11-15	11-11-15
H	321	0	321	0	11-11-15	11-11-15
G	677	0	677	0	11-11-15	11-11-15

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED		SNOW	LIVE			
F	173	95/0	40/0	0/0	0/0	38/0	0/0
B	142	108/0	14/0	0/0	0/0	22/0	0/0
H	278	131/0	79/0	0/0	0/0	68/0	0/0
G	550	315/0	119/0	0/0	0/0	116/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)		
FR-TO		FROM TO			FR-TO				
A-B	0 / 14	-84.3	-84.3	0.03 (1)	10.00	H-C	-196 / 0	0.03 (1)	
B-J	-32 / 0	-84.3	-84.3	0.01 (1)	6.25	C-G	-27 / 0	0.01 (1)	
J-C	-49 / 0	-84.3	-84.3	0.05 (1)	6.25	G-D	-512 / 0	0.07 (1)	
C-D	0 / 0	-84.3	-84.3	0.34 (1)	10.00	G-E	0 / 0	0.00 (1)	
D-E	0 / 0	-84.3	-84.3	0.34 (1)	10.00	I-J	-121 / 0	0.00 (1)	
F-E	-158 / 0	0.0	0.0	0.02 (1)	7.81				
B-I	0 / 38	-28.0	-28.0	0.05 (1)	10.00				
I-H	0 / 38	-28.0	-28.0	0.09 (2)	10.00				
H-G	0 / 25	-28.0	-28.0	0.17 (2)	10.00				
G-F	0 / 0	-28.0	-28.0	0.17 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.34 (C-D:1), BC=0.17 (G-H:2), WB=0.07 (D-G:1), SSI=0.20 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1668

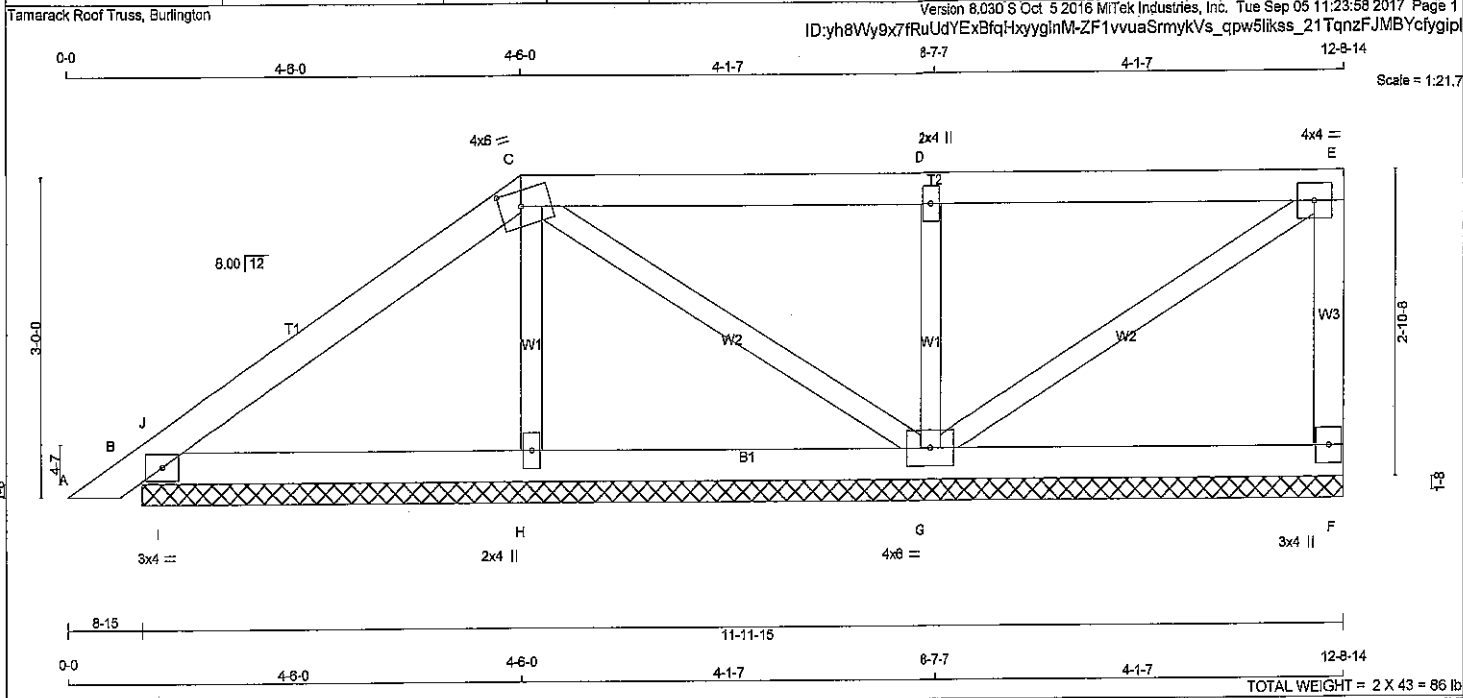
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (D) (INPUT = 0.90)
JSI METAL= 0.09 (D) (INPUT = 1.00)



DWG NO. TAM 4494617
STRUCTURAL
COMPONENT ONLY



LUMBER
 N. L. G. A. RULES
 CHORDS SIZE LUMBER DESCR.
 A - C 2x4 DRY No.2 SPF
 C - E 2x4 DRY No.2 SPF
 F - E 2x4 DRY No.2 SPF
 B - F 2x4 DRY No.2 SPF
 ALL WEBS 2x3 DRY No.2 SPF
 DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-J	MT20	3.0	4.0		
C	TTWW-m	MT20	4.0	6.0	1.75	2.50
D	TMW-w	MT20	2.0	4.0		
E	TMW-w	MT20	4.0	4.0		
F	BMV1-p	MT20	3.0	4.0		
G	BMWWW1-t	MT20	4.0	6.0		
H	BMW1-w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD IN-SX
	VERT	HORZ	DOWN	HORZ		
F	181	0	181	0	0	11-11-15
B	301	0	301	0	0	11-11-15
H	328	0	328	0	0	11-11-15
G	592	0	592	0	0	11-11-15

UNFACTORED REACTIONS

JT	COMBINED	MAX./MIN. COMPONENT REACTIONS				
		SNOW	LIVE	PERM. LIVE	WIND	DEAD
F	150	81/0	36/0	0/0	0/0	33/0
B	232	153/0	37/0	0/0	0/0	42/0
H	285	133/0	82/0	0/0	0/0	70/0
G	478	281/0	97/0	0/0	0/0	98/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINTS (F), B, H, G

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0/14	-84.3 -84.3	0.03 (1)	10.00	H-C	-176/0 0.03 (1)	
B-J	-73/0	-84.3 -84.3	0.04 (1)	6.25	C-G	-75/0 0.03 (1)	
J-C	-91/0	-84.3 -84.3	0.15 (1)	6.25	G-D	-433/0 0.07 (1)	
C-D	-4/0	-84.3 -84.3	0.24 (1)	10.00	G-E	0/5 0.00 (8)	
D-E	-4/0	-84.3 -84.3	0.24 (1)	10.00	I-J	-283/74 0.00 (1)	
F-E	-133/0	0.0	0.02 (1)	7.81			
B-I	0/71	-28.0 -28.0	0.13 (1)	10.00			
I-H	0/71	-28.0 -28.0	0.13 (1)	10.00			
H-G	0/64	-28.0 -28.0	0.12 (2)	10.00			
G-F	0/0	-28.0 -28.0	0.12 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 25.6 PSF
 DL = 3.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. G/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 088-09
 - TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.24 (C-D:1), BC=0.13 (H-I:1), WB=0.07 (D-G:1), SSI=0.22 (B-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 622 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches
 PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (D) (INPUT = 0.90)
 JSI METAL = 0.08 (D) (INPUT = 1.00)



DWG NO. TAM 4494717
 STRUCTURAL
 COMPONENT ONLY

LUL/LUS/LJS/HUS/HHUS/HGUS



Standard and Double-Shear Joist Hangers



This product is preferable to similar connectors because of
a) easier installation, b) higher capacities, c) lower installed
cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 258–259.

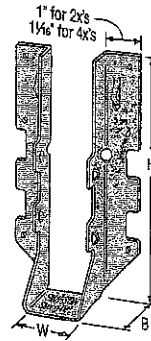
Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 20–24.

Installation:

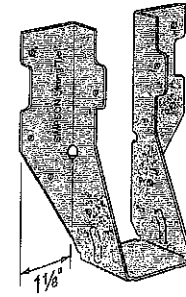
- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1 1/2" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

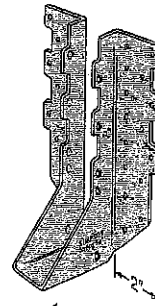
- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on p. 126.



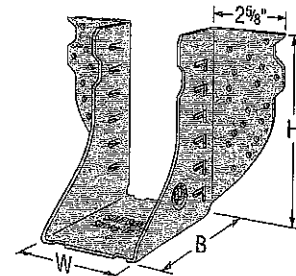
✓ LUS28



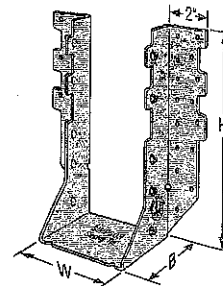
LU26L



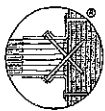
✓ HUS210
(HUS26, HUS28,
and HHUS similar)



✓ HGUS28-2



✓ HHUS210-2



Double-Shear
Nailing
Top View

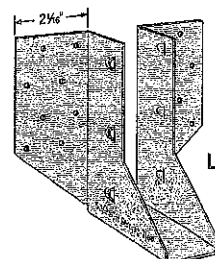
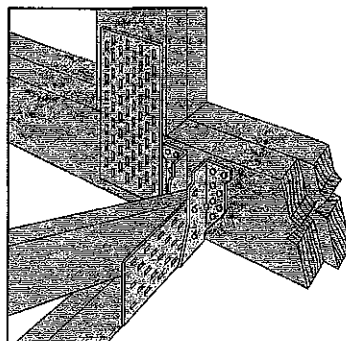


Double-Shear
Nailing
Side View;
Do not
bend tab



Dome Double-Shear
Nailing Side View
(available on
some models)
U.S. Patent 5,603,580

**Typical HUS26
Installation
with Reduced
Heel Height**
(Truss Designer
to provide
fastener quantity
for connecting
multiple members
together)



LJS26DS

Plated Truss Connectors

LUL/LUS/LJS/HUS/HHUS/HGUS

HHUS/HGUS

See Hanger Options information on pp. 125–127.

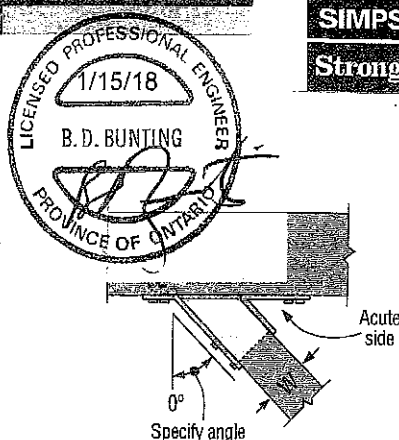
HHUS — Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.62 of the table value
- The joist must be bevel-cut to allow for double-shear nailing

HGUS — Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Joist	Down Resistance	Uplift
$W < 2"$	Bevel or square cut	0.62 of table value	0.46 of table value
$2" < W < 6"$	Bevel cut	0.67 of table value	0.41 of table value
$2" < W < 6"$	Square cut	0.46 of table value	0.41 of table value
$W > 6"$	Bevel cut	0.75 of table value	0.41 of table value



**Top View HHUS Hanger
Skewed Right**
(Joist must be bevel cut)
All joist nails installed on the
outside angle (non-acute side).

Standard and Double-Shear Joist Hangers (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance			
		W	H	B	d _e ³	Header	Joist	D-Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
								(K _D = 1.15) lb.	(K _D = 1.00) lb.	(K _D = 1.15) lb.	(K _D = 1.00) lb.
Single 2x Sizes											
LUS24	18	1½	3½	1¾	2¼	(4) 10d	(2) 10d	710	1625	645	1155
								3.16	7.23	2.87	5.14
LU24L	22	1½	3	1½	2¼	(4) 10d	(2) 10d x 1 ½"	380	1020	320	725
								1.60	4.54	1.42	3.22
LU26L	22	1½	5	1½	4¾	(6) 10d	(4) 10d x 1 ½"	720	1605	645	1140
								3.20	7.14	2.87	5.07
LUS26	18	1½	4¾	1¾	3¾	(4) 10d	(4) 10d	1420	2170	1290	1630
								6.32	9.65	5.74	7.25
HUS26	16	1½	5¾	3	3¼	(14) 16d	(6) 16d	2705	4940	2085	3875
								11.30	21.97	9.20	17.24
LJS26DS	18	1½	5	3½	4¾	(16) 16d	(6) 16d	2055	4265	1460	4115
								9.14	18.97	6.49	18.31
HGUS26	12	1½	5¾	5	4¼	(20) 16d	(8) 16d	2685	6625	2685	5700
								11.96	29.51	11.96	25.35
LU26L	20	1½	6¾	1½	5¾	(8) 10d	(6) 10d x 1 ½"	1140	2185	1020	1550
								5.07	9.72	4.54	6.89
LUS28	18	1½	6¾	1¾	3¾	(8) 10d	(4) 10d	1420	2520	1290	1790
								6.32	11.21	5.74	7.96
HUS28	16	1½	7¼	3	6¼	(22) 16d	(8) 16d	3605	5365	2675	4345
								16.04	23.86	11.90	19.33
HGUS28	12	1½	7¼	5	6¾	(36) 16d	(12) 16d	3310	7675	3310	6900
								14.74	34.19	14.74	30.73
LU210L	20	1½	8	1½	7¾	(10) 10d	(6) 10d x 1 ½"	1140	2495	1020	1770
								5.07	11.10	4.54	7.87
LUS210	18	1½	7¼	1¾	3¾	(8) 10d	(4) 10d	1420	2785	1290	2210
								6.32	12.39	5.74	9.83

1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.

2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.

3. d_e is the distance from the bearing seat to the top joist nail.

4. Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-C-N10TRSSCN and/or see installation notes.

5. Nails: 16d = 0.162" dia. x 3½" long. See pp. 27–28 for other nail sizes and information.

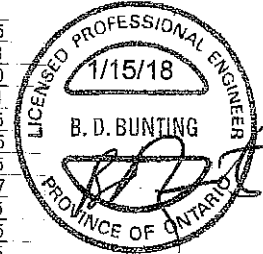
Face-Mount Hangers

SIMPSON
Strong-Tie

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

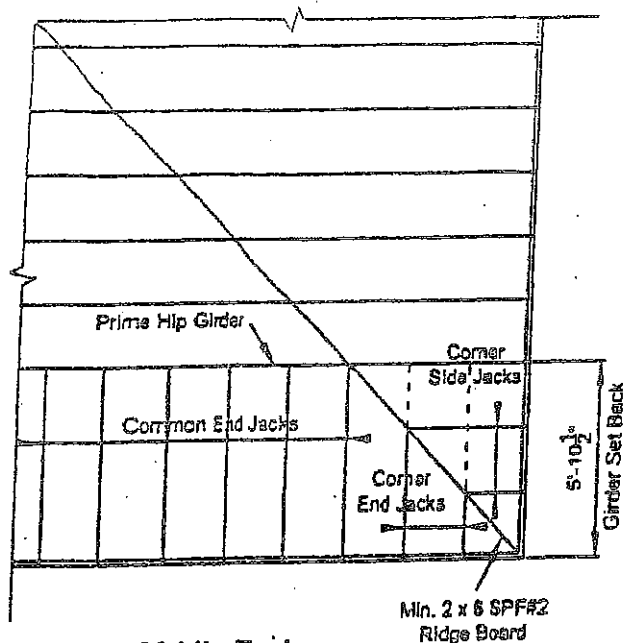
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance			
		W	H	B	d _o ²	Header	Joist	D-Flr-L		S-P-F	
								Uplift	Normal	Uplift	Normal
								(K _D = 1.15)	(K _D = 1.00)	(K _D = 1.15)	(K _D = 1.00)
								lb.	lb.	lb.	lb.
								kN	kN	kN	kN
Double 2x Sizes											
LUS24-2	18	3 1/8	3 1/8	2	1 1/2	(4) 16d	(2) 16d	835	2020	590	1435
LUS26-2	18	3 1/8	4 1/8	2	4	(4) 16d	(4) 16d	3.71	8.99	2.62	6.38
HHUS26-2	14	3 1/8	5 1/8	3	3 1/8	(14) 16d	(6) 16d	1720	2595	1545	1920
HGUS26-2	12	3 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	7.65	11.54	6.87	8.54
LUS28-2	18	3 1/8	7	2	4	(8) 16d	(4) 16d	2850	7335	2065	5205
HHUS28-2	14	3 1/8	7 1/8	3	6 1/8	(22) 16d	(8) 16d	12.68	32.63	9.20	23.15
HGUS28-2	12	3 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	4385	8950	3110	6355
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	19.51	39.81	13.83	28.27
HHUS210-2	14	3 1/8	9 1/8	3	8	(30) 16d	(10) 16d	7.65	14.79	6.87	11.45
HGUS210-2	12	3 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	3765	8940	2675	6345
								16.75	39.77	11.90	28.22
								6070	12980	4310	9215
								27.00	57.74	19.17	40.99
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
HHUS210-2	14	3 1/8	9 1/8	3	8	(30) 16d	(10) 16d	11.48	20.02	10.32	14.21
HGUS210-2	12	3 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	4670	9660	4235	7000
								20.77	42.97	18.84	31.14
								6840	14015	4855	10270
								30.43	62.34	21.60	45.69
Triple 2x Sizes											
HGUS26-3	12	4 1/8	5 1/2	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
HGUS28-3	12	4 1/8	7 1/4	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
HHUS210-3	14	4 1/8	9	3	7 1/8	(30) 16d	(10) 16d	4670	9670	4235	8865
								20.77	43.02	18.84	30.54
HGUS210-3	12	4 1/8	9 1/4	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400
								30.43	65.14	21.60	46.26
Quadruple 2x Sizes											
HGUS26-4	12	6 1/8	5 3/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
HGUS28-4	12	6 1/8	7 3/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
HHUS210-4	14	6 1/8	8 3/8	3	7 3/8	(30) 16d	(10) 16d	4670	10155	4235	7210
								20.77	45.17	18.84	32.07
HGUS210-4	12	6 1/8	9 3/8	4	8 3/8	(46) 16d	(16) 16d	6840	14645	4855	10400
								30.43	65.14	21.60	46.26
HGUS212-4	12	6 1/8	10 3/8	4	10 3/8	(56) 16d	(20) 16d	7640	14995	5425	10645
								33.98	66.70	24.13	47.35
HGUS214-4	12	6 1/8	12 3/8	4	11 3/8	(66) 16d	(22) 16d	10130	16400	7195	11645
								45.06	72.95	32.00	51.80
4x Sizes											
LUS46	18	3 1/8	4 1/8	2	3 1/8	(4) 18d	(4) 16d	1720	2595	1545	1920
HHUS46	14	3 1/8	5 1/8	3	3 1/8	(14) 16d	(6) 16d	7.65	11.54	6.87	8.54
HGUS46	12	3 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	2540	7335	2065	5205
LUS48	18	3 1/8	6 1/8	2	3 1/8	(6) 16d	(4) 16d	11.30	32.63	9.20	23.15
HHUS48	14	3 1/8	7 1/8	3	6 1/8	(22) 16d	(8) 16d	4385	8950	3110	6355
HGUS48	12	3 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	19.51	39.81	13.83	28.27
LUS410	18	3 1/8	8 1/8	2	5 1/8	(8) 16d	(6) 16d	1720	3325	1545	2575
HHUS410	14	3 1/8	9 1/8	3	8 1/8	(30) 16d	(10) 16d	7.65	14.79	6.87	11.45
HGUS410	12	3 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	3765	8940	2675	6345
								16.75	39.77	11.90	28.22
								6070	12980	4310	9215
								27.00	57.74	19.17	40.99
LUS412	18	3 1/8	10 1/8	2	6 1/8	(12) 16d	(6) 16d	2580	4500	2320	3195
HGUS412	12	3 1/8	10 1/8	4	10 1/8	(56) 16d	(20) 16d	11.48	20.02	10.32	14.21
HGUS414	12	3 1/8	12 1/8	4	11 1/8	(66) 16d	(22) 16d	6840	14015	4855	10270
								30.43	62.34	21.60	45.69
								7640	14995	5425	10645
								33.98	66.70	24.13	47.35
								10130	16400	7195	11645
								45.06	72.95	32.00	51.80



Plated Truss Connectors

MICRO CITY ENGINEERING SERVICES INC.

TEL: (519) 287-2242



45° Hip End

LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2

BOTTOM CHORD : 2 x 4 SPF#2

WEBS : 2 x 3 SPF#2

UNLESS OTHERWISE SHOWN

DESIGN LOAD:

TOP CHORD LIVE LOAD : 34.8 P.S.F.

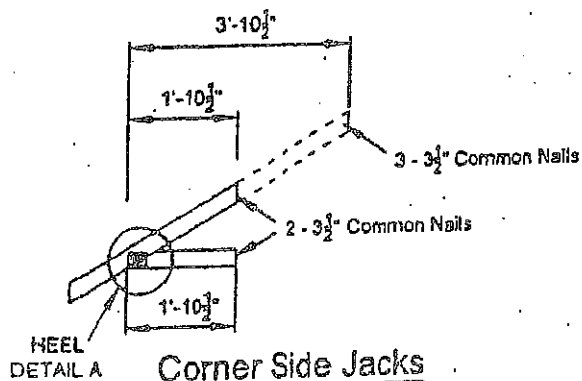
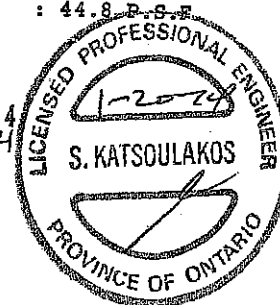
TOP CHORD DEAD LOAD : 3.0 P.S.F.

BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.

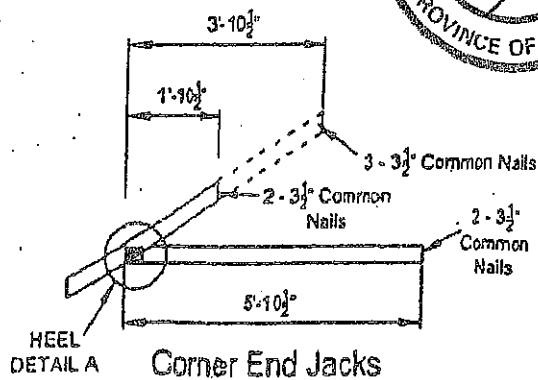
BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD : 44.8 P.S.F.

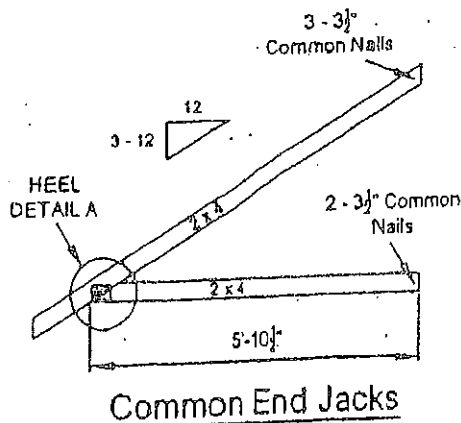
DWG NO TAM 3495.14
STRUCTURAL
COMPONENT ONLY



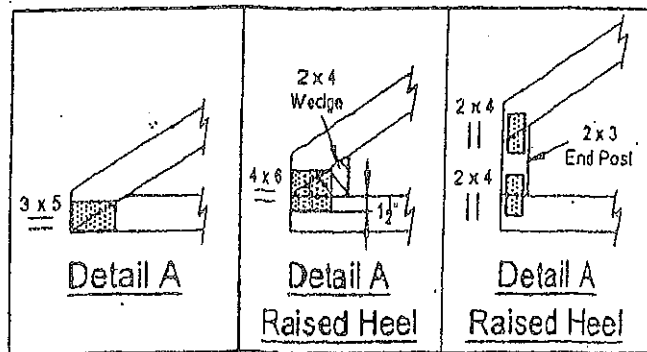
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

MICRO CITY ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

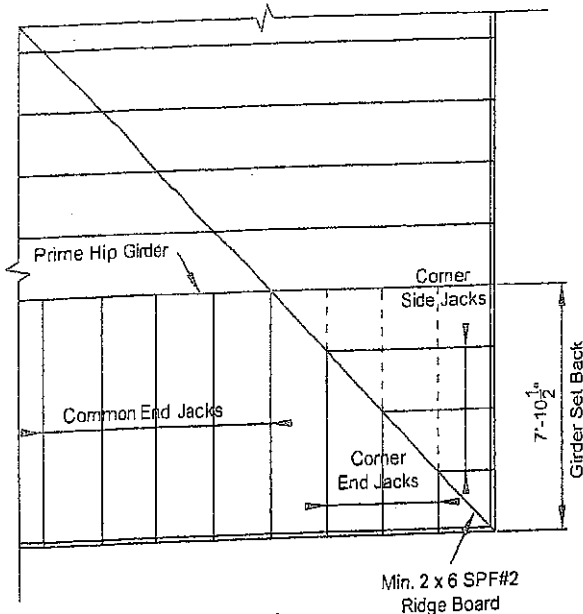
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

DESIGN LOAD:

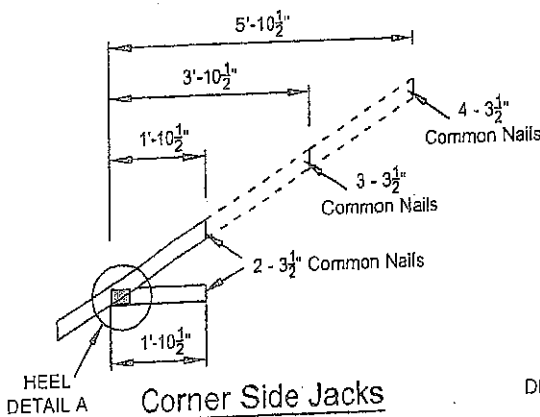
TOP CHORD LIVE LOAD : 40.5 P.S.F.
TOP CHORD DEAD LOAD : 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD : 50.5 P.S.F.

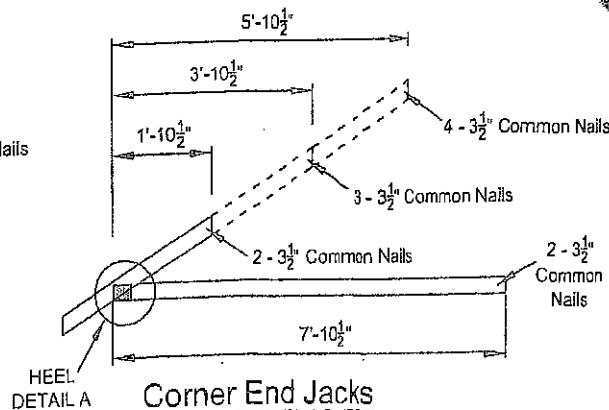


45° Hip End

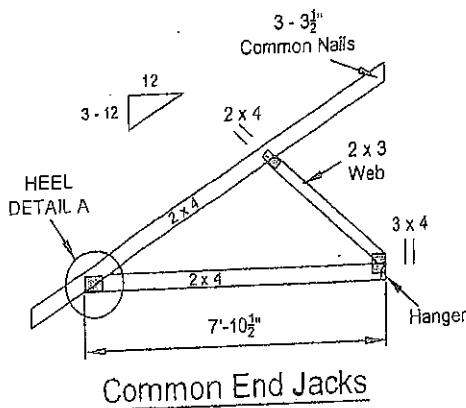
DWG NO TAM 3504.1
STRUCTURAL
COMPONENT ONLY



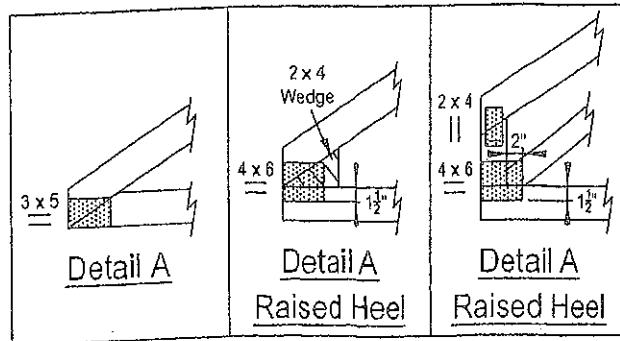
Corner Side Jacks



Corner End Jacks



Common End Jacks



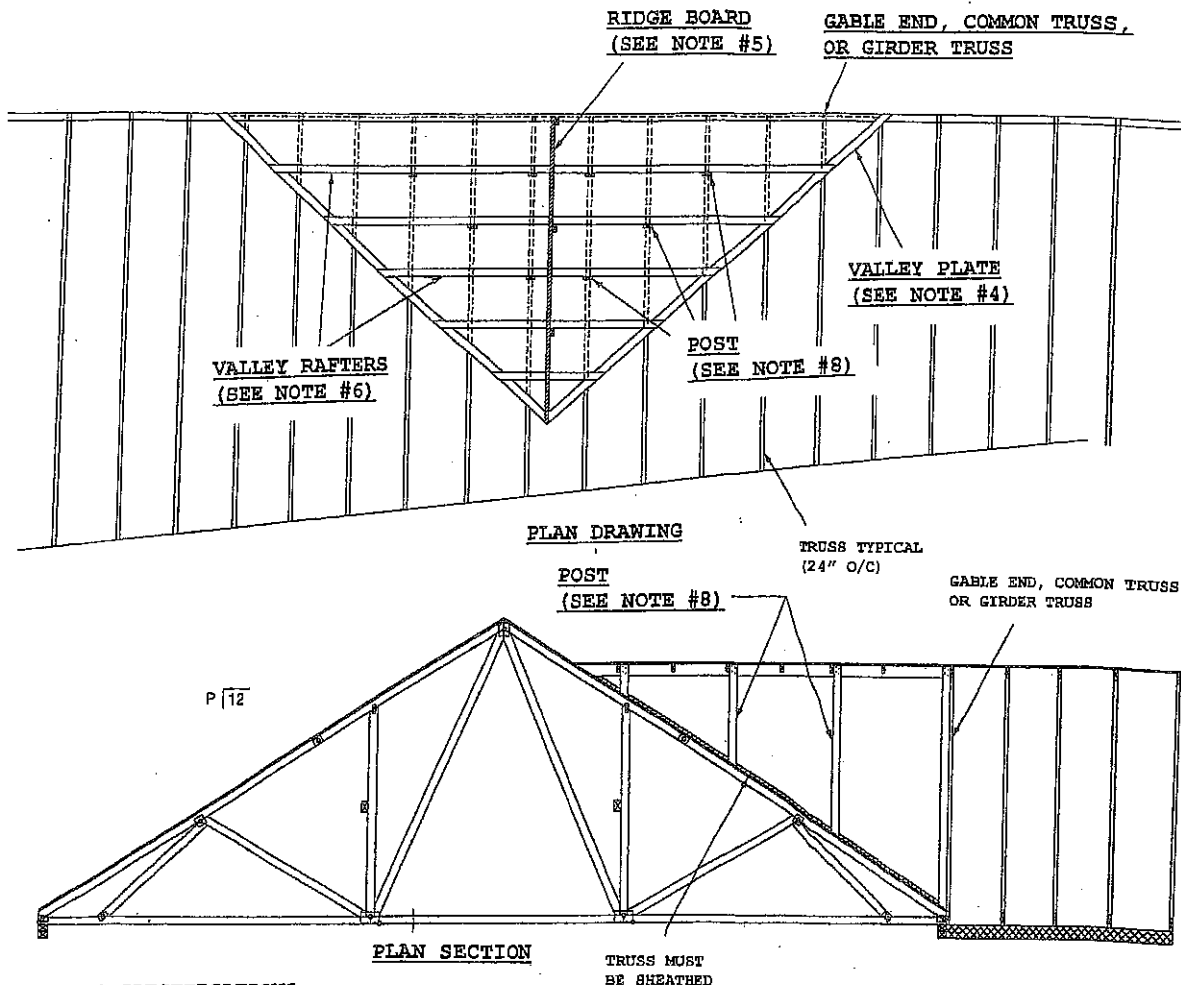
NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

MICRO CITY ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

CONVENTIONAL VALLEY FRAMING DETAIL



GENERAL SPECIFICATIONS:

- (1) WITH THE BASE TRUSSES ERECTED (INSTALLED), APPLY SHEATHING TOP CHORD OF SUPPORTING (BASE) TRUSSES.
- (2) BRACE BOTTOM CHORD AND WEB MEMBERS AS PER PRE-ENGINEERED TRUSS DESIGNS.
- (3) DEFINE VALLEY RIDGE BY RUNNING A LEVEL STRING FROM THE INTERSECTING RIDGE OF THE (a) GABLE END, (b) GIRDER TRUSS OR (c) COMMON TRUSS TO THE ROOF SHEATHING.
- (4) INSTALL 2 X 6 VALLEY PLATES ON FLAT. FASTEN TO EACH SUPPORTING TRUSS WITH (2) 16d (3.5" X 0.131") NAILS.
- (5) SET A 2 X 6 #2 RIDGE BOARD (MAX. 10'-0" RIDGE) OR 2 X 8 #2 SPF RIDGE BOARD (MAX. 20'-0" RIDGE). SUPPORT RIDGE BOARD WITH 2 X 4 POSTS SPACED 48" O/C. BEVEL BOTTOM OF POST TO SET EVENLY ON THE SHEATHING. FASTEN POST TO RIDGE WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST TO ROOF SHEATHING WITH (3) 10d (3" X 0.131") TOE-NAILS.
- (6) FRAME VALLEY RAFTERS FROM VALLEY PLATE TO RIDGE BOARD. MAXIMUM RAFTER SPACING IS 24" O/C. FASTEN VALLEY RAFTER TO RIDGE BEAM WITH (3) 16d (3.5" X 0.131") TOE-NAILS. FASTEN VALLEY RAFTER TO VALLEY PLATE WITH (3) 16d (3.5" X 0.131") TOE-NAILS.
- (7) SUPPORT THE VALLEY RAFTERS WITH 2 X 4 POSTS AT 48" O/C (OR LESS) ALONG EACH RAFTER. INSTALL POSTS IN A STAGGERED PATTERN AS SHOWN ON PLAN DRAWING. ALIGN POSTS WITH TRUSSES BELOW. FASTEN VALLEY RAFTER TO POST WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST THROUGH SHEATHING TO SUPPORTING TRUSSES WITH (2) 16d (3.5" X 0.131") NAILS.
- (8) POSTS SHALL BE 2 X 4 #2 SPF OR BETTER. POSTS EXCEEDING 75" IN HEIGHT SHALL BE INCREASED TO 4 X 4 #2 SPF, OR BETTER, OR BE PRE-ASSEMBLED TWO (2) PLY 2 X 4 #2 SPF OR BETTER FASTENED TOGETHER WITH 2 ROWS OF 10d (3" X 0.131") NAILS AT 6" O/C.
- (9) MAINTAIN A MINIMUM 3/4" LUMBER EDGE DISTANCE WHEN NAILING. NAIL SPACING SHOULD APPROXIMATE A MINIMUM 1-3/4" O/C OR MORE UNLESS NOTED OTHERWISE. ALL CONSTRUCTION TO CONFORM TO ONTARIO BUILDING CODE (CURRENT ADDITION) AT ALL TIMES.

NOTES:

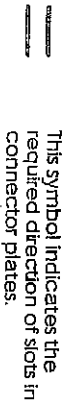
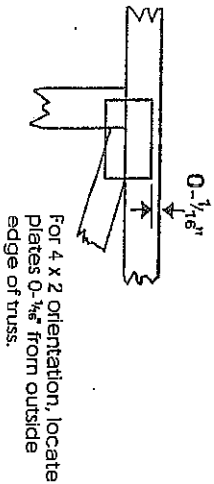
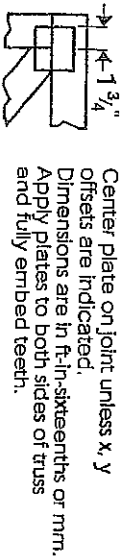
- (10) 48" O/C (MAXIMUM POST SPACING).
- (11) ROOF LIVE LOAD = 34.8 PSF (MAX.)
- (12) ROOF DEAD LOAD = 10.0 PSF (MAX.)
- (13) PART 9 APPLICATION ONLY (ONTARIO BUILDING CODE)
- (14) PART 4 APPLICATION ONLY (ONTARIO BUILDING CODE) WITH APPROVED REVIEW BY LICENSED PROFESSIONAL ENGINEER.
- (15) BASE TRUSS SPACING (24" O/C MAX.)
- (16) ALL PRE-ENGINEERED BASE TRUSS COMPONENTS TO BE SEALED BY LICENSED PROFESSIONAL ENGINEER AND THIS DETAIL TO BE VERIFIED AND APPROVED BY SAME WHEN RIDGE BOARD LENGTH EXCEEDS 12'-0".
- (17) ALL BASE TRUSSES: P = 4 (4/12) - MINIMUM.
- (18) ALL VALLEY RAFTERS: P = 4 (4/12) - MINIMUM.



DWG NO T&M 6305.14
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Symbols

PLATE LOCATION AND ORIENTATION



*Plate location details available in Mitek software or upon request.

PLATE SIZE

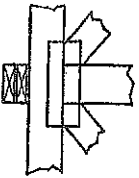
4 X 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

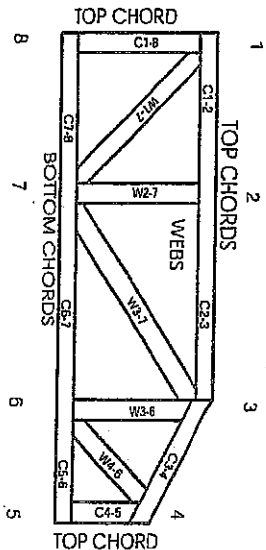
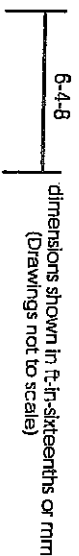


BEARING



Industry Standards:
TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:
T1996-L, T0319-L, T13270-L, T2691-R

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Mitek Engineering Reference Sheet: Mill-7473C rev. 10-08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative I, L, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

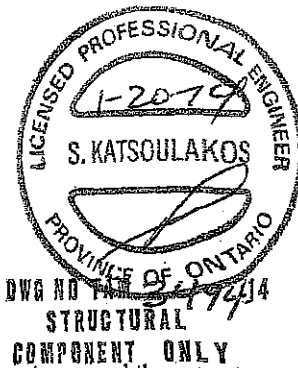
Micro City Engineering Services Inc.
(BCIN: 26064; FIRM BCIN: 29991)

RR #1, Po Box 61

Glencoe, Ontario

NOL 1M0

(519) 287 - 2242; Fax: (519) 287 - 5750 (Call)



Responsibilities:

Micro City Engineering Services is responsible for the design of trusses as individual components.

It is the responsibilities of others to ascertain that the design loads utilized on this (these) drawing(s) meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdiction over such decisions.

All dimensions are to be verified by the owner, contractor, architect, or other authority having input over such decisions prior to truss component manufacture. At no time shall Micro City Engineering Services Inc. or its employees be responsible for dimension errors.

Micro City Engineering Services Inc. bears no responsibility for the erection of any truss components. Persons erecting truss components are cautioned to seek professional advice regarding temporary and permanent bracing systems and to be **totally** familiar with all aspects of truss erection prior to proceeding on **any** truss component erection job. Any bracing shown on Micro City Engineering Services Inc. or Tamarack Roof Trusses Inc. sealed or unsealed truss component drawings is specified for the single truss component in question and is identified as an integral part of the design for that particular truss component but is **not** meant to represent the only required bracing for that particular truss component when installed as a component in a series of truss components in a roof truss system.

It is the truss manufacturer's responsibility to ensure that trusses are manufactured in accordance with Micro City Engineering Services Inc. specifications outlined below:

SPECIFICATIONS:

Truss components, sealed by Micro City Engineering Services Inc. must conform to the relevant sections of the current Building Code of Ontario and Canada (Part 4 or Part 9) or the current Farm Building Code of Canada in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the Truss Plate Institute of Canada (TPIC). All unit lumber and nailing stresses identified on truss component design drawings and/or used in the design of individual truss components shall conform to the current CSA Wood Design standard identified in the current Building Code and TPIC Design Standards.

The lumber used to manufacture any truss component is to conform to the specified size and grade identified on the truss drawing.

The lumber used in the manufacture of any truss component is not to exceed 19% during its service use unless specifically noted on the truss drawing.

The lumber used in the manufacture of any truss component is not to be treated with any chemicals during its service life unless specifically noted on the truss drawing.

Connector plates shall be applied to both faces of the truss component at each joint and shall be positioned exactly as specified.

The top chord of any truss component is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the sealed truss component drawing but not exceeding 24" o/c (Part 9 design) and not exceeding 48" o/c (Part 4 or Agricultural design).

When a truss component is to be installed with no rigid ceiling attached directly to the bottom chord, then the bottom chord is to be laterally braced at intervals not exceeding 3m (or 10'-0").

All sealed or unsealed truss component drawings provided by Micro City Engineering Services Inc. Or Tamarack Roof Trusses Inc. should be read in conjunction with the following:

Warning-Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473C rev 10-'08 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer - not the truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpik.ca and BCSI Building Component Safety Information available from the Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA, 22314.