

HIGHGROVE-5E
EL:2 UNIT1
285994

HIGHGROVE-2
EL:2 (REV)- UNIT2
285478

HIGHGROVE-2
EL:2 - UNIT3
285996

HIGHGROVE-1
EL:2 (REV)-UNIT4
285997

HIGHGROVE-12
EL:2 (REV)- UNIT5
285998

ASPHALT SHINGLES
12" FINISH O.H.
R.T.M.C
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:

HGUS26-2 -(XX)
LJS26DS -(V)
LUS24 -(O)

8/12 PITCHES (TYP.)
UNLESS NOTED

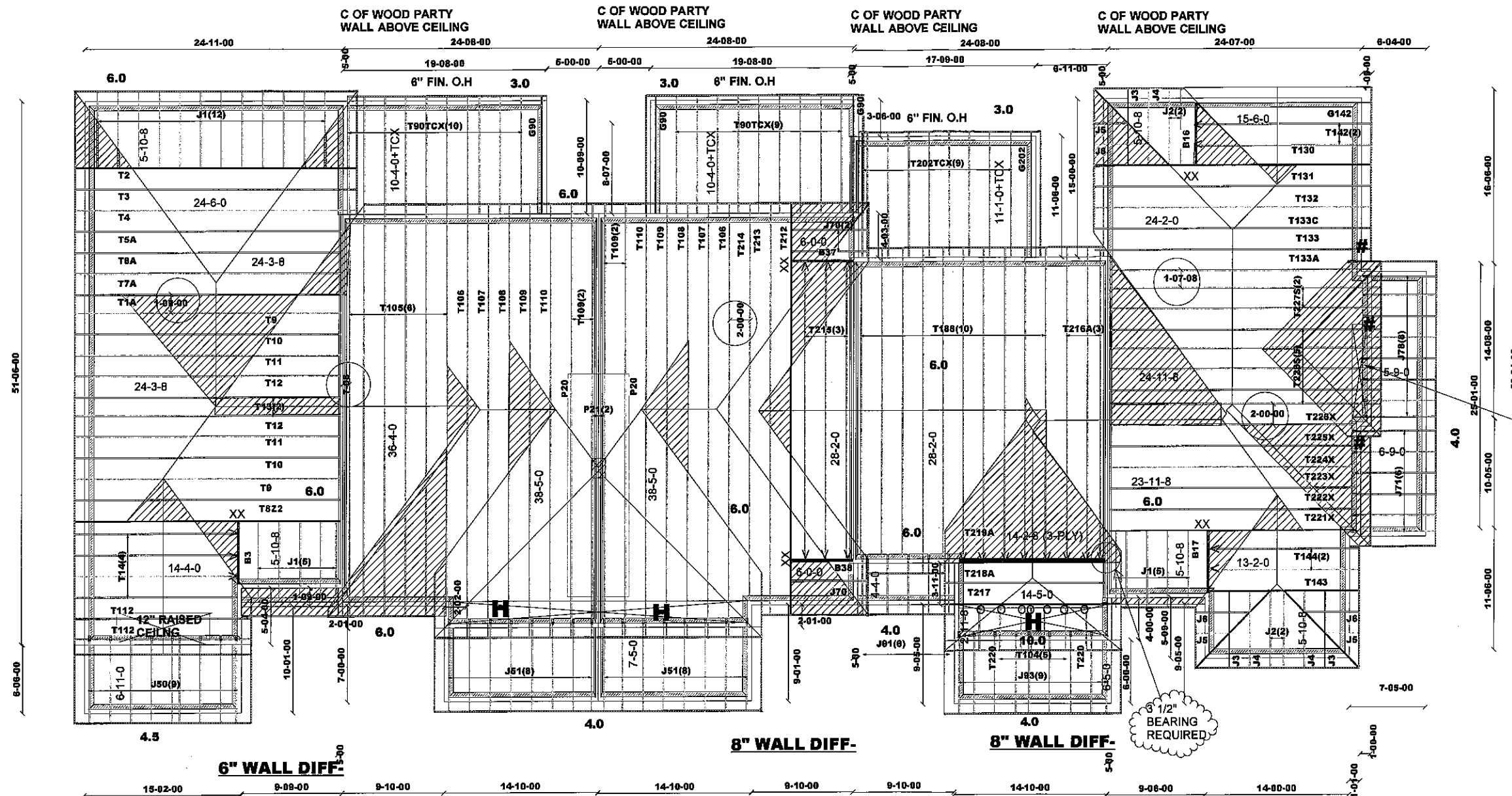
DENOTES:

H -2'-0"-0 HIGHER PLATE
-1'-6"-0 RAISED FASCIA

ALL B- 2-2X10 BEAMS (FLUSH)

18" HIGHER PLATE AREA

T-170634
DENOTES:
CONVENTIONAL
FRAMING



ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC LATEST EDITION
ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"o.c.
WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.
POSTS LONGER THAN 6' TO BE Laterally BRACED SO THAT THE DISTANCE
BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN LOADS:

SNOW LOAD 25.6 PSF
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

m11,009



Job Track: 43954
Layout ID: 285993
Plan Log: 93446

Builder / Location:

GREENPARK HOMES / WATERDOWN

Project: RUSSEL GARDENS PHASE 2

Date: 4/19/2018

Designer: sonny

Model / Elevation:

BLOCK217 / UNIT1T05 (REVISED)

Mitek ver 7.5.0

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Delivery Shiplist

DATE	04/19/18
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 285994 LOCATION:
 BUILDER: GREENPARK - RUSSELL GARDENS PH2 SUB-BUILDER:
 MODEL: HIGHGROVE5E ELEVATION: 2- UNIT1

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	T1A COMMON	8.00 0.00	24-03-08	09-06-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	102.41 64.00		
	1	T2 HIP GIRDER	8.00 0.00	24-06-00	04-01-04	2 X 6	2 X 6	01-03-08 01-01-00	01-04-13 01-04-13	145.29 89.67		
	1	T3 HIP	8.00 0.00	24-06-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	97.09 60.83		
	1	T4 HIP	8.00 0.00	24-06-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	105.79 67.00		
	1	T5A HIP	8.00 0.00	24-03-08	07-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	104.32 67.00		
	1	T6A HIP	8.00 0.00	24-03-08	08-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	108.82 69.50		
	1	T7A HIP	8.00 0.00	24-03-08	09-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	114.67 72.50		
	1	T8Z2 HALF HIP	8.00 0.00	24-03-08	04-01-04	2 X 6	2 X 6	01-03-08 00-00-00	01-04-13 04-01-04	143.26 88.34		
	2	T9 HALF HIP	8.00 0.00	24-03-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-01-04	197.10 123.66		
	2	T10 HALF HIP	8.00 0.00	24-03-08	06-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 06-01-04	212.66 133.34		
	2	T11 HALF HIP	8.00 0.00	24-03-08	07-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 07-01-04	225.56 142.68		
	2	T12 HALF HIP	8.00 0.00	24-03-08	08-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 08-01-04	234.64 146.34		
	2	T13 HALF HIP	8.00 0.00	24-03-08	09-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 09-01-04	245.76 151.66		
	4	T14 COMMON	8.00 0.00	14-04-00	06-02-02	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	247.04 156.00		
	2	T112 ROOF	8.00 0.00	14-04-00	06-02-02	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	135.80 89.00		
	17	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	285.43 181.39		
	9	J50 JACK	4.50 0.00	06-11-00	02-11-12	2 X 4	2 X 4	01-03-08 00-00-00	00-04-10 02-11-12	184.86 114.03		

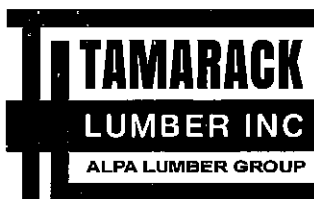
TOTAL # TRUSS= 50.00

TOTAL BFT OF ALL TRUSSES=

1816.94 BFT. TOTAL WEIGHT OF ALL TRUSSES= 2890.50 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
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Delivery Shiplist

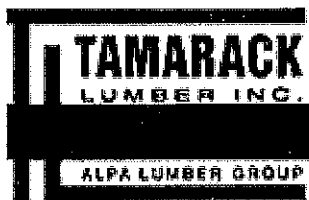
DATE	04/19/18
SALES REP	Mario

JOB TRACK: 43954	LAYOUT ID: 285994	LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH 2	SUB-BUILDER:	
MODEL: HIGHGROVE5E	ELEVATION: 2- UNIT1	

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	HGUS26-2	
3	Hangers	LJS26DS	

TOTAL # ITEMS= 4.00



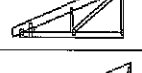
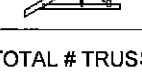
Delivery Shiplist

DATE	09/08/17
SALES REP	Mario

JOB TRACK:43954	LAYOUT ID: 285478 D	LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH	SUB-BUILDER:	
MODEL: HIGHGROVE2	ELEVATION:	2(REV)-UNIT2

ROOF TRUSSES

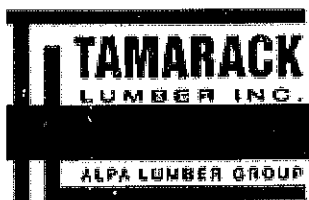
ROOF TRUSS SPACING:24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	6	T105 ROOF	6.00 0.00	36-04-00	09-09-12	2 X 4	2 X 6	01-03-08 01-03-08	01-02-00 01-02-00	1043.58 655.02		
	1	T106 ROOF	6.00 0.00	38-05-00	09-04-13	2 X 4	2 X 6	01-03-08 01-03-08	01-02-00 01-04-13	210.94 130.00		
	1	T107 ROOF	6.00 0.00	38-05-00	10-03-00	2 X 4	2 X 6	01-03-08 01-03-08	01-02-00 01-04-13	200.19 125.33		
	1	T108 ROOF	6.00 0.00	38-05-00	10-03-00	2 X 4	2 X 6	01-03-08 01-03-08	01-02-00 01-04-13	203.82 125.67		
	3	T109 HIP	6.00 0.00	38-05-00	08-06-13	2 X 4	2 X 6	01-03-08 01-03-08	01-02-00 01-04-13	624.12 381.99		
	1	T110 HIP	6.00 0.00	38-05-00	09-10-13	2 X 4	2 X 6	01-03-08 01-03-08	01-02-00 01-04-13	221.07 137.50		
	10	T90TCX MONOPITCH	3.00 0.00	10-04-00	03-05-09	2 X 6	2 X 4	00-09-08 00-00-00	00-08-04 03-05-09	449.90 303.30		
	1	G90 MONOPITCH	3.00 0.00	10-04-00	03-03-04	2 X 6	2 X 4	00-09-08 00-00-00	00-08-04 03-03-04	40.90 26.67		
	1	P20 PIGGYBACK	6.00 0.00	14-02-02	02-06-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-03 00-04-07	44.32 27.50		
	1	P21 PIGGYBACK	6.00 0.00	14-02-02	03-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-03 00-04-07	45.00 28.67		
	8	J51 JACK TRUSS	4.00 0.00	07-05-00	02-09-10	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-09-10	177.36 116.00		

TOTAL # TRUSS= 34.00

TOTAL BFT OF ALL TRUSSES=

2057.65 BFT. TOTAL WEIGHT OF ALL TRUSSES= 3261.20 LBS.



Delivery Shiplist

DATE	09/08/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 285996 LOCATION:
 BUILDER: GREENPARK - RUSSELL GARDENS PH2 SUB-BUILDER:
 MODEL: HIGHGROVE2 ELEVATION: 2-UNIT3

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	T212 HIP GIRDER	6.00 0.00	36-04-00	05-04-13	2 X 4	2 X 6	01-03-08 01-03-08	01-02-00 01-02-00	216.32 132.00		
	1	T213 HIP	6.00 0.00	36-04-00	06-08-13	2 X 4	2 X 6	01-03-08 01-03-08	01-02-00 01-02-00	168.56 104.00		
	1	T214 HIP	6.00 0.00	36-04-00	08-00-13	2 X 4	2 X 6	01-03-08 01-03-08	01-02-00 01-02-00	171.29 106.00		
	1	T106 ROOF	6.00 0.00	38-05-00	09-04-13	2 X 4	2 X 6	01-03-08 01-03-08	01-02-00 01-04-13	210.94 130.00		
	1	T107 ROOF	6.00 0.00	38-05-00	10-03-00	2 X 4	2 X 6	01-03-08 01-03-08	01-02-00 01-04-13	200.19 125.33		
	1	T108 ROOF	6.00 0.00	38-05-00	10-03-00	2 X 4	2 X 6	01-03-08 01-03-08	01-02-00 01-04-13	203.82 125.67		
	3	T109 HIP	6.00 0.00	38-05-00	08-06-13	2 X 4	2 X 6	01-03-08 01-03-08	01-02-00 01-04-13	624.12 381.99		
	1	T110 HIP	6.00 0.00	38-05-00	09-10-13	2 X 4	2 X 6	01-03-08 01-03-08	01-02-00 01-04-13	221.07 137.50		
	3	T215 COMMON	6.00 0.00	28-02-00	07-06-08	2 X 4	2 X 4	00-00-00 00-00-00	00-06-00 00-06-00	280.20 172.50		
	9	T90TCX MONOPITCH	3.00 0.00	10-04-00	03-05-09	2 X 6	2 X 4	00-09-08 00-00-00	00-08-04 03-05-09	404.91 272.97		
	2	G90 MONOPITCH	3.00 0.00	10-04-00	03-03-04	2 X 6	2 X 4	00-09-08 00-00-00	00-08-04 03-03-04	81.80 53.34		
	1	P20 PIGGYBACK	6.00 0.00	14-02-02	02-06-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-03 00-04-07	44.32 27.50		
	1	P21 PIGGYBACK	6.00 0.00	14-02-02	03-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-03 00-04-07	45.00 28.67		
	8	J51 JACK TRUSS	4.00 0.00	07-05-00	02-09-10	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-09-10	176.80 116.00		
	3	J70 JACK-OPEN	8.00 0.00	06-00-00	05-04-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 00-05-03	54.42 32.01		

TOTAL # TRUSS= 37.00

TOTAL BFT OF ALL TRUSSES=

1945.48 BFT.

TOTAL WEIGHT OF ALL TRUSSES=

3103.76 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
6	Hangers	LJS26DS	

TOTAL # ITEMS= 8.00



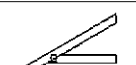
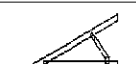
Delivery Shiplist

DATE	09/08/17
SALES REP	Mario

JOB TRACK:43954 LAYOUT ID: 285997 LOCATION:
 BUILDER: GREENPARK - RUSSELL GARDENS PH 2 SUB-BUILDER:
 MODEL: HIGHGROVE 1 ELEVATION: 2(REV)-UNIT4

ROOF TRUSSES

ROOF TRUSS SPACING:24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	10	T188 COMMON	6.00 0.00	28-02-00	08-02-08	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	1101.60 686.70		
	3	T216A COMMON	6.00 0.00	28-02-00	08-08-08	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 02-02-00	332.25 208.50		
	1	T217 HIP GIRDER	8.00 0.00	14-05-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	03-04-13 03-04-13	72.45 46.00		
	1	T218A HIP	8.00 0.00	14-02-08	07-09-04	2 X 4	2 X 4	01-03-08 00-00-00	03-04-13 03-06-08	81.11 52.00		
	1 3 Ply	T219A COMMON	8.00 0.00	14-02-08	08-02-08	2 X 6	2 X 6	00-00-00 00-00-00	03-04-13 03-06-08	305.01 183.99		
	5	T104 MONO TRUSS	10.00 0.00	02-11-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 06-01-04	98.55 64.15		
	2	T220 HALF HIP	10.00 0.00	02-11-08	02-08-13	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 04-08-13	41.08 29.34		
	9	T202TCX MONOPITCH	3.00 0.00	11-01-00	03-07-13	2 X 6	2 X 4	00-09-08 00-00-00	00-08-04 03-07-13	431.37 279.00		
	1	G202 GABLE	3.00 0.00	11-01-00	03-05-08	2 X 6	2 X 4	00-09-08 00-00-00	00-08-04 03-05-08	43.96 29.33		
	6	J91 MONOPITCH	4.00 0.00	04-04-00	01-09-04	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 01-09-04	70.86 48.00		
	9	J93 MONOPITCH	4.00 0.00	06-05-00	02-05-10	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-05-10	163.44 105.03		

TOTAL # TRUSS= 50.00

TOTAL BFT OF ALL TRUSSES=

1732.04 BFT. TOTAL WEIGHT OF ALL TRUSSES= 2741.68 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
8	Hangers	LJS26DS	
7	Hangers	LUS24	

TOTAL # ITEMS= 15.00



Delivery Shiplist

DATE	09/08/17
SALES REP	Mario

JOB TRACK: 43954	LAYOUT ID: 285998	LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH 2	SUB-BUILDER:	
MODEL: HIGHGROVE 12	ELEVATION: 2(REV)- UNITS	

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY PLY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
						TOP	BOT					
	1	T130 HIP GIRDER	8.00 0.00	24-02-00	05-03-13	2 X 6	2 X 6	01-03-08 01-03-08	01-04-13 01-04-13	144.06 88.67		
	1	T131 HIP	8.00 0.00	24-02-00	06-07-13	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	103.24 67.00		
	1	T132 HIP	8.00 0.00	24-02-00	07-11-13	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	109.56 69.50		
	1	T133 COMMON	8.00 0.00	24-02-00	09-05-08	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	103.85 64.17		
	1	T133C COMMON	8.00 0.00	24-02-00	09-03-13	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	103.67 64.17		
	1	T133A COMMON	8.00 0.00	23-11-08	09-05-08	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	101.57 63.50		
	1	T221X HALF HIP	8.00 0.00	23-11-08	04-01-04	2 X 6	2 X 6	00-00-00 00-00-00	00-08-02 04-01-04	126.20 75.34		
	1	T222X HALF HIP	8.00 0.00	23-11-08	05-01-04	2 X 4	2 X 4	00-00-00 00-00-00	00-08-02 05-01-04	100.70 64.33		
	1	T223X HALF HIP	8.00 0.00	23-11-08	06-01-04	2 X 4	2 X 4	00-00-00 00-00-00	00-08-02 06-01-04	107.80 66.83		
	1	T224X HALF HIP	8.00 0.00	23-11-08	07-01-04	2 X 4	2 X 4	00-00-00 00-00-00	00-08-02 07-01-04	105.97 65.67		
	1	T225X HALF HIP	8.00 0.00	23-11-08	08-01-04	2 X 4	2 X 4	00-00-00 00-00-00	00-08-02 08-01-04	112.36 69.33		
	1	T226X HALF HIP	8.00 0.00	23-11-08	09-01-04	2 X 4	2 X 4	00-00-00 00-00-00	00-08-02 09-01-04	120.83 74.83		
	2	T227S ROOF	8.00 0.00	24-11-08	09-05-08	2 X 4	2 X 4	01-03-08 00-00-00	02-10-13 03-01-00	231.10 147.34		
	5	T228S ROOF	8.00 0.00	24-11-08	09-05-08	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 03-01-00	618.85 398.35		
	2	T142 COMMON	8.00 0.00	15-06-00	06-06-13	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	132.28 84.00		
	1	G142 GABLE	8.00 0.00	15-06-00	06-06-13	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	66.17 42.00		
	1	T143 HIP GIRDER	8.00 0.00	13-02-00	05-03-13	2 X 4	2 X 6	01-03-08 01-03-08	01-04-13 01-04-13	71.46 45.33		
	2	T144 COMMON	8.00 0.00	13-02-00	05-09-08	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	114.76 74.00		



Delivery Shiplist

DATE	09/08/17
SALES REP	Mario

JOB TRACK:43954 LAYOUT ID: 285998 LOCATION:
 BUILDER: GREENPARK - RUSSELL GARDENS PH2 SUB-BUILDER:
 MODEL: HIGHGROVE 12 ELEVATION: 2(REV)- UNIT5

ROOF TRUSSES

ROOF TRUSS SPACING:24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	5	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	83.95 53.35		
	4	J2 JACK-OPEN	8.00 0.00	05-10-08	05-03-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-03-13	71.32 42.68		
	3	J3 JACK-OPEN	8.00 0.00	05-10-08	02-07-02	2 X 4	2 X 4	01-03-08 -04-01-01	01-04-13 00-03-08	50.52 30.99		
	3	J4 JACK-OPEN	8.00 0.00	05-10-08	03-11-02	2 X 4	2 X 4	01-03-08 -02-01-01	01-04-13 00-03-08	58.74 36.99		
	3	J5 JACK-OPEN	8.00 0.00	01-10-08	02-07-02	2 X 4	2 X 4	01-03-08 -00-01-01	01-04-13 00-03-08	27.51 17.01		
	3	J6 JACK-OPEN	8.00 0.00	01-10-08	03-11-02	2 X 4	2 X 4	01-03-08 01-10-15	01-04-13 00-05-03	35.73 23.01		
	6	J71 JACK	4.00 0.00	06-09-00	02-06-15	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-06-15	117.18 73.02		
	8	J78 MONOPITCH	4.00 0.00	05-09-00	02-02-15	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-02-15	121.04 74.64		

TOTAL # TRUSS= 60.00

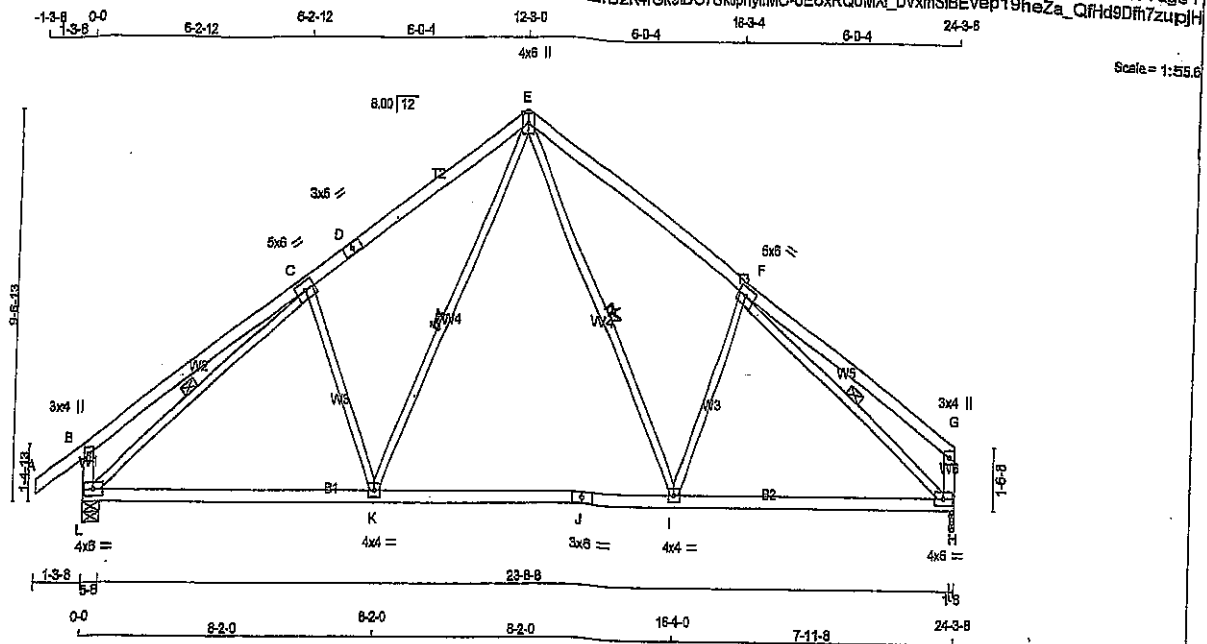
TOTAL BFT OF ALL TRUSSES=

1976.05 BFT. TOTAL WEIGHT OF ALL TRUSSES= 3140.42 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
4	Hangers	LJS26DS	

TOTAL # ITEMS= 6.00



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESOR.
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
L - B	2x4	DRY	No.2
H - G	2x4	DRY	No.2
L - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW+t	MT20	5.0	6.0		
D	TS-t	MT20	3.0	6.0		
E	TTVW+p	MT20	4.0	6.0	Edge	
F	TMVW-t	MT20	5.0	6.0		
G	TMV+p	MT20	3.0	4.0		
H	BMVW-t	MT20	4.0	6.0		
I	BMVW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMVW-t	MT20	4.0	4.0		
L	BMVW-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
L	1479	0	1479	0	5-8	5-8
H	1363	0	1363	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LC CASE MAX/MIN COMPONENT REACTIONS						
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1198	892/0	255/0	0/0	0/0	251/0	0/0
H	1119	621/0	255/0	0/0	0/0	243/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.98 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-L, F-H. *E, K, E, I*

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	MAX. FACTORED CSI (LC)
FR-TO				FR-TO			
A-B	0/32	-84.3	-84.3 0.11 (1)	C-K	-352/44	10.00	0.19 (1)
B-C	0/34	-84.3	-84.3 0.52 (1)	K-E	0/632	10.00	0.14 (1)
C-D	-1409/0	-84.3	-84.3 0.45 (1)	E-I	0/589	4.98	0.13 (1)
D-E	-1409/0	-84.3	-84.3 0.45 (1)	I-F	-318/39	4.98	0.17 (1)
E-F	-1388/0	-84.3	-84.3 0.44 (1)	F-H	-1653/0	5.01	0.71 (1)
F-G	0/34	-84.3	-84.3 0.50 (1)	L-C	-1653/0	10.00	0.87 (1)
G-H	-311/0	0.0	0.0 0.03 (1)				
H-I	-155/0	0.0	0.0 0.02 (1)				
L-K	0/1275	-28.0	-28.0 0.58 (2)				
K-J	0/689	-28.0	-28.0 0.54 (2)				
J-I	0/689	-28.0	-28.0 0.54 (2)				
I-H	0/1245	-28.0	-28.0 0.56 (2)				

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 25.6 PSF
 DL = 3.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 088-09
 - TPC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.81")
 CALCULATED VERT. DEFL.(LL) = $L/999$ (0.17")
 ALLOWABLE DEFL.(TL) = $L/360$ (0.81")
 CALCULATED VERT. DEFL.(TL) = $L/969$ (0.28")

CSI: TC=0.52/1.00 (B-C:1), BC=0.58/1.00 (K-L:2), WB=0.71/1.00 (C-L:1), SS=0.21/1.00 (C-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 616 354 1657 522 2284 1655

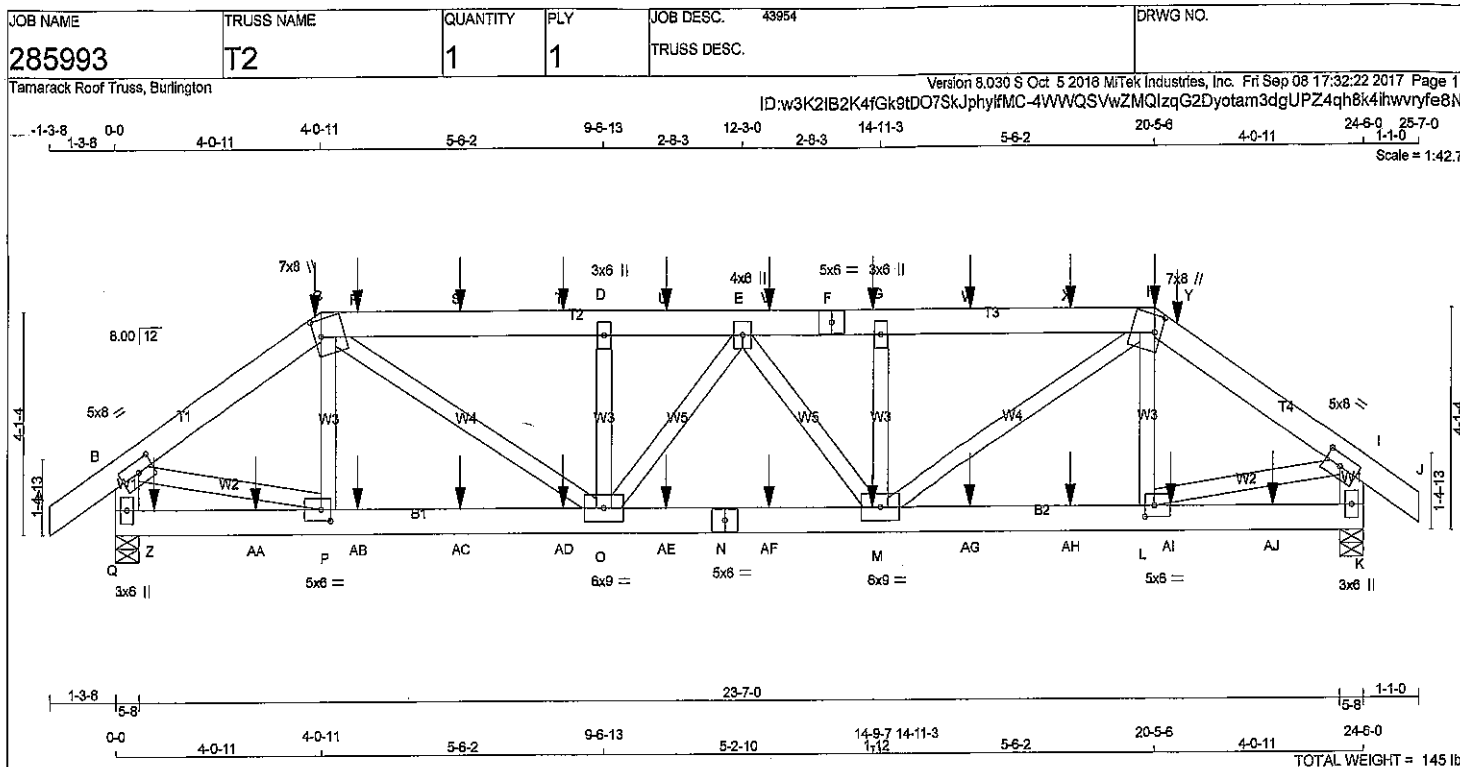
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (L) (INPUT = 0.90)
 JSI METAL= 0.43 (C) (INPUT = 1.00)



DRWG NO. TAMB3984-18
 STRUCTURAL
 COMPONENT ONLY

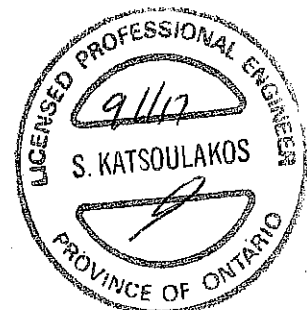


LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. A - C 2x8 DRY No.2 SPF C - F 2x6 DRY No.2 SPF F - H 2x6 DRY No.2 SPF H - J 2x6 DRY No.2 SPF Q - B 2x6 DRY No.2 SPF K - I 2x8 DRY No.2 SPF Q - N 2x8 DRY No.2 SPF N - K 2x8 DRY No.2 SPF ALL WEBS 2x4 DRY No.2 SPF EXCEPT DRY: SEASONED LUMBER.				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS FACTORED GROSS REACTION MAXIMUM FACTORED GROSS REACTION INPUT REGRD JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX Q 2441 0 2441 0 0 5-8 5-8 K 2455 0 2455 0 0 5-8 5-8 UNFACTORED REACTIONS 1ST LCASE MAX/MIN COMPONENT REACTIONS JT COMBINED SNOW LIVE PERM. LIVE WIND DEAD SOIL Q 1997 1121 / 0 446 / 0 0 / 0 0 / 0 430 / 0 0 / 0 K 1994 1142 / 0 430 / 0 0 / 0 0 / 0 421 / 0 0 / 0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.96 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (4) CHORDS WEBS MAX. FACTORED MAX. FACTORED MEMB. FORCE VERT. LOAD LC1 MAX MAX. MEMB. MAX. FORCE MAX. (LBS) (PLF) CSI (LC) UNBRAC (LBS) CSI (LC) FR-TO LENGTH FR-TO A-B 0 / 33 -84.3 -84.3 0.07 (1) 10.00 P-C -234 / 201 0.04 (1) B-C -2812 / 0 -84.3 -84.3 0.17 (1) 4.82 C-O 0 / 1872 0.33 (1) C-R -3889 / 0 -84.3 -84.3 0.40 (1) 3.96 O-D -720 / 0 0.12 (1) R-S -3889 / 0 -84.3 -84.3 0.40 (1) 3.96 M-G -717 / 0 0.12 (1) S-T -3889 / 0 -84.3 -84.3 0.40 (1) 3.96 M-H 0 / 1835 0.32 (1) T-D -3889 / 0 -84.3 -84.3 0.40 (1) 3.96 L-H -247 / 197 0.04 (1) D-U -3889 / 0 -84.3 -84.3 0.31 (1) 4.08 B-P 0 / 2387 0.42 (1) U-E -3889 / 0 -84.3 -84.3 0.31 (1) 4.08 L-I 0 / 2434 0.43 (1) E-V -3903 / 0 -84.3 -84.3 0.29 (1) 4.08 O-E -77 / 0 0.02 (1) V-F -3903 / 0 -84.3 -84.3 0.29 (1) 4.08 E-M -52 / 0 0.01 (1) F-G -3903 / 0 -84.3 -84.3 0.29 (1) 4.08 G-W -3903 / 0 -84.3 -84.3 0.39 (1) 3.97 W-X -3903 / 0 -84.3 -84.3 0.39 (1) 3.97 X-H -3903 / 0 -84.3 -84.3 0.39 (1) 3.97 H-Y -2838 / 0 -84.3 -84.3 0.19 (1) 4.77 Y-I -2838 / 0 -84.3 -84.3 0.19 (1) 4.77 I-J 0 / 28 -84.3 -84.3 0.05 (1) 10.00 Q-B -2358 / 0 0.0 0.0 0.17 (1) 6.66 K-I -2391 / 0 0.0 0.0 0.17 (1) 6.62 Q-Z 0 / 0 -28.0 -28.0 0.14 (3) 10.00 Z-AA 0 / 0 -28.0 -28.0 0.14 (3) 10.00 AA-P 0 / 0 -28.0 -28.0 0.14 (3) 10.00 P-AB 0 / 2325 -28.0 -28.0 0.38 (1) 10.00 AB-AC 0 / 2325 -28.0 -28.0 0.38 (1) 10.00 AC-AD 0 / 2325 -28.0 -28.0 0.38 (1) 10.00 AD-O 0 / 2325 -28.0 -28.0 0.38 (1) 10.00 O-AE 0 / 5934 -28.0 -28.0 0.80 (1) 10.00 AE-N 0 / 5934 -28.0 -28.0 0.80 (1) 10.00 N-AF 0 / 5934 -28.0 -28.0 0.80 (1) 10.00 AF-M 0 / 5934 -28.0 -28.0 0.80 (1) 10.00 M-AG 0 / 2370 -28.0 -28.0 0.38 (1) 10.00 AG-AH 0 / 2370 -28.0 -28.0 0.38 (1) 10.00 AH-L 0 / 2370 -28.0 -28.0 0.38 (1) 10.00 L-AI 0 / 0 -28.0 -28.0 0.14 (3) 10.00 AI-AJ 0 / 0 -28.0 -28.0 0.14 (3) 10.00 AJ-K 0 / 0 -28.0 -28.0 0.14 (3) 10.00				DESIGN CRITERIA *** SPECIAL LOADS ANALYSIS *** GEOMETRY AND/OR BASIC LOADS CHANGED BY USER. LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE SPECIFIED LOADS: TOP CH. LL = 25.6 PSF DL = 3.0 PSF BOT CH. LL = 10.5 PSF DL = 7.0 PSF TOTAL LOAD = 46.1 PSF SPACING = 24.0 IN. C/C LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12 *** NON STANDARD GIRDER *** ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES. THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010 THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012, BCBC 2012, ABC 2014 - CSA 086-09 - TPIC 2011 (55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL)= L/360 (0.82") CALCULATED VERT. DEFL.(LL) = L/999 (0.13") ALLOWABLE DEFL.(TL)= L/360 (0.82") CALCULATED VERT. DEFL.(TL) = L/999 (0.20") CSI: TC=0.40 (C-D:1), BC=0.60 (M-O:1), WB=0.43 (H-L:1), SS=0.29 (C-D:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00 COMPANION LIVE LOAD FACTOR = 0.50 AUTOSOLVE HEELS OFF TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 618 354 1657 922 2284 1656 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.88 (L) (INPUT = 0.90) JSI METAL= 0.69 (N) (INPUT = 1.00)			
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DRG NO. TAM 45618.81
STRUCTURAL
COMPONENT ONLY

FACTORED CONCENTRATED LOADS (LBS)						
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.
C	4-0-11	-23	-25	—	FRONT	VERT
C	4-0-11	-222	-222	—	FRONT	VERT
G	14-9-4	-101	-101	—	BACK	VERT
H	20-5-5	-23	-25	—	FRONT	VERT
H	20-5-5	-222	-222	—	FRONT	VERT
M	14-9-4	-40	-70	—	BACK	VERT
R	4-9-4	-113	-113	—	BACK	VERT
S	6-9-4	-101	-101	—	BACK	VERT
T	8-9-4	-101	-101	—	BACK	VERT
U	10-9-4	-101	-101	—	BACK	VERT
V	12-9-4	-101	-101	—	BACK	VERT
W	16-9-4	-101	-101	—	BACK	VERT
X	18-9-4	-101	-101	—	BACK	VERT
Y	20-9-4	-130	-130	—	BACK	VERT
Z	9-4	-43	-76	—	BACK	VERT
AA	2-9-4	-40	-70	—	BACK	VERT
AB	4-9-4	-40	-70	—	BACK	VERT
AC	6-9-4	-40	-70	—	BACK	VERT
AD	8-9-4	-40	-70	—	BACK	VERT
AE	10-9-4	-40	-70	—	BACK	VERT
AF	12-9-4	-40	-70	—	BACK	VERT
AG	16-9-4	-40	-70	—	BACK	VERT
AH	18-9-4	-40	-70	—	BACK	VERT
AI	20-9-4	-40	-70	—	BACK	VERT
AJ	22-9-4	-40	-70	—	BACK	VERT

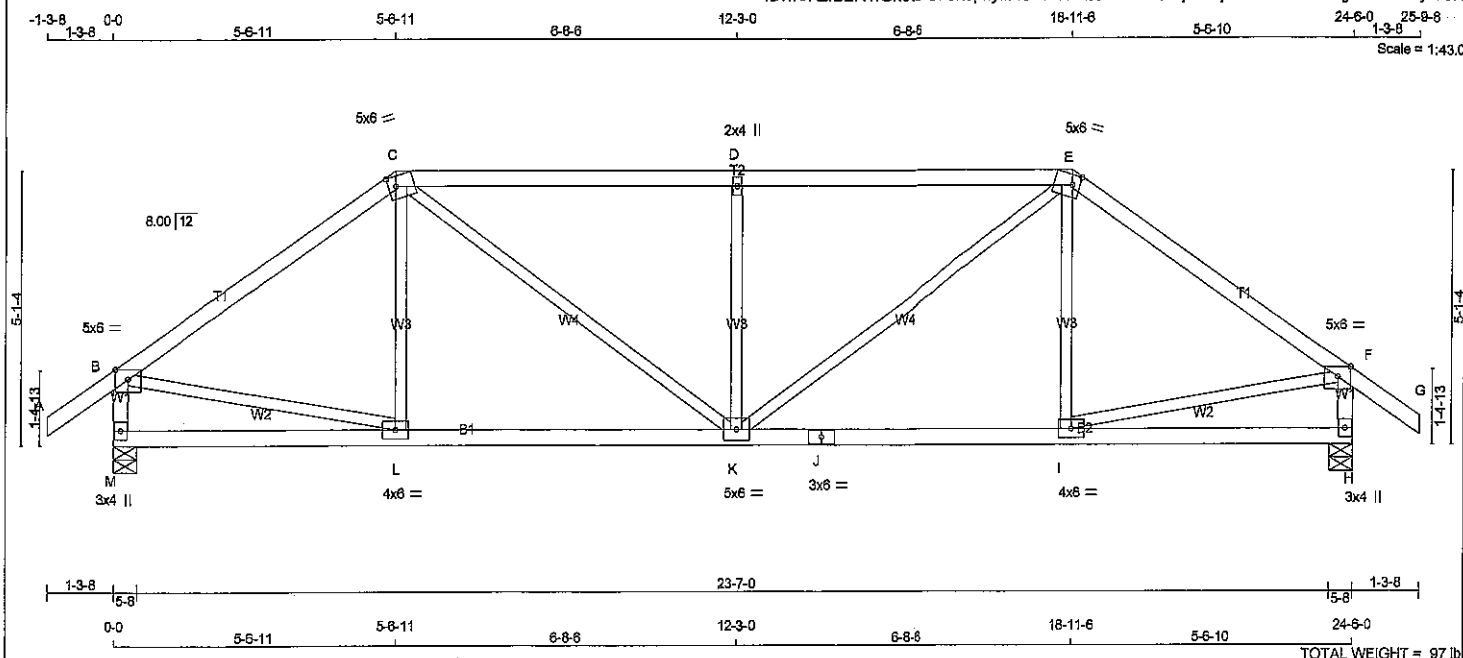


OWNED BY TAM 45618.17
 STRUCTURAL
 COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
285993	T3	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Fri Sep 08 17:32:22 2017 Page 1
ID:w3K2lB2K4fGk9tDO7SkJphyfMC-4VWVQSvWZMQlZqG2Dyotam3daFPcEqjNk4ihwvryfe8N



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
M - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
M - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
SPF			
DRY, SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVV-p	MT20	5.0	6.0	Edge
C	TTWW-m	MT20	5.0	6.0	2.25 1.75
D	TMW-w	MT20	2.0	4.0	
E	TTWW-m	MT20	5.0	6.0	2.25 1.75
F	TMVV-p	MT20	5.0	6.0	Edge
H	BMV1+p	MT20	3.0	4.0	
I	BMWW-t	MT20	4.0	6.0	
J	BS-t	MT20	3.0	6.0	
K	BMWW-t	MT20	5.0	6.0	
L	BMWW-t	MT20	4.0	6.0	
M	BMV1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	DOWN	GROSS REACTION	DOWN	BRG	IN-SX	BRG	IN-SX
M	1491	0	1491	0	0	5-8	5-8	5-8	5-8
H	1491	0	1491	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1208	697 / 0	257 / 0	0 / 0	0 / 0	253 / 0	0 / 0
H	1208	697 / 0	257 / 0	0 / 0	0 / 0	253 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.85 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	FORCE (LBS)	FACTORED MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 32	-84.3 -84.3	0.11 (1)	10.00	L-C	-47 / 210	0.05 (3)
B-C	-1511 / 0	-84.3 -84.3	0.55 (1)	4.67	C-K	0 / 747	0.17 (1)
C-D	-1862 / 0	-84.3 -84.3	0.80 (1)	3.85	K-E	-692 / 0	0.27 (1)
D-E	-1862 / 0	-84.3 -84.3	0.80 (1)	3.85	E-F	0 / 747	0.17 (1)
E-F	-1511 / 0	-84.3 -84.3	0.55 (1)	4.67	F-G	-47 / 210	0.05 (3)
F-G	0 / 32	-84.3 -84.3	0.11 (1)	10.00	G-L	0 / 1278	0.28 (1)
M-B	-1430 / 0	0.0 0.0	0.15 (1)	8.83	L-F	0 / 1278	0.28 (1)
H-F	-1430 / 0	0.0 0.0	0.15 (1)	8.83			
M-L	0 / 0	-28.0 -28.0	0.25 (3)	10.00			
L-K	0 / 1255	-28.0 -28.0	0.40 (2)	10.00			
K-J	0 / 1255	-28.0 -28.0	0.40 (2)	10.00			
J-I	0 / 1255	-28.0 -28.0	0.40 (2)	10.00			
I-H	0 / 0	-28.0 -28.0	0.25 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.82")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (0.82")
CALCULATED VERT. DEFL.(TL) = L/999 (0.14")

CSI: TC=0.80 (C-D:1), BC=0.40 (K-L:2), WB=0.29 (B-L:1), SSI=0.27 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

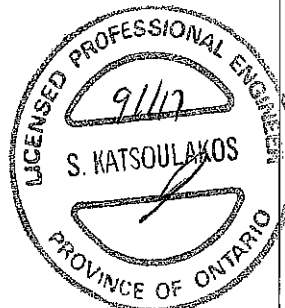
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1857 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (L) (INPUT = 0.90)
JSI METAL= 0.37 (J) (INPUT = 1.00)



DRWG NO. TAM45619-17
STRUCTURAL
COMPONENT ONLY

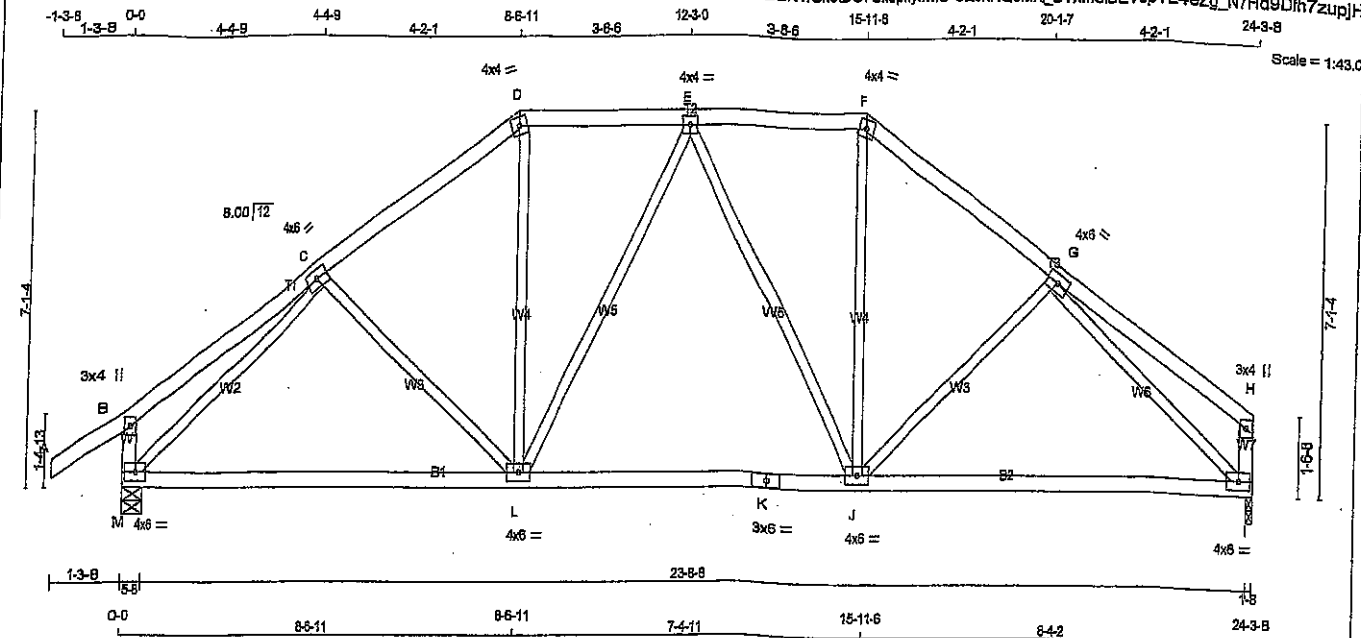
Tamarack Roof Truss, Burlington



OWG NO. TAM 4562017
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285994	T5A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESIGN
A - D	2x4 DRY	No.2	SPF		
D - F	2x4 DRY	No.2	SPF		
F - H	2x4 DRY	No.2	SPF		
M - H	2x4 DRY	No.2	SPF		
I - H	2x4 DRY	No.2	SPF		
M - K	2x4 DRY	No.2	SPF		
K - I	2x4 DRY	No.2	SPF		
ALL WEBS	2x3 DRY	No.2	SPF		
EXCEPT					
DRY: SEASONED LUMBER.					

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	4.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TMVW-t	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	4.0		
G	TMVW-t	MT20	4.0	6.0		
H	TMV+p	MT20	3.0	4.0		
I	BMVW-t	MT20	4.0	6.0		
J	BMVW-t	MT20	4.0	6.0		
K	BS-t	MT20	3.0	6.0		
L	BMVW-t	MT20	4.0	6.0		
M	BMVW-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQD
JT	VERT	HORZ	DOWN	HORZ
M	1479	0	1479	0
I	1363	0	1363	0

UNFACTORED REACTIONS

1ST CASE	MAX/MIN. COMPONENT REACTIONS
JT	COMBINED
M	1188
I	1119

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.32 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	FORCE	FACTORED	VERT. LOAD	MAX	CS	UNBRAC	LENGTH	FR-TO	WEBS	MEMB.	FORCE	FACTORED	VERT. LOAD	MAX	CS	UNBRAC	LENGTH	FR-TO
A-B	0/32	-84.3	-84.3	0.11 (1)	10.00	C-L	-117/85	0.07 (1)											
B-C	0/22	-84.3	-84.3	0.24 (1)	10.00	L-D	0/495	0.11 (1)											
C-D	-1388/0	-84.3	-84.3	0.21 (1)	5.82	L-E	-195/0	0.24 (1)											
D-E	-1142/0	-84.3	-84.3	0.15 (1)	5.79	E-J	-225/0	0.27 (1)											
E-F	-1128/0	-84.3	-84.3	0.15 (1)	5.82	J-F	0/484	0.11 (1)											
F-G	-1371/0	-84.3	-84.3	0.20 (1)	5.35	J-G	-70/102	0.04 (1)											
G-H	0/22	-84.3	-84.3	0.23 (1)	10.00	M-C	-1680/0	0.93 (1)											
H-B	-255/0	0.0	0.0	0.08 (1)	7.81	G-I	-1632/0	0.88 (1)											
I-H	-131/0	0.0	0.0	0.01 (1)	7.81														
M-L	0/1222	-28.0	-28.0	0.57 (2)	10.00														
L-K	0/1233	-28.0	-28.0	0.55 (2)	10.00														
K-J	0/1233	-28.0	-28.0	0.68 (2)	10.00														
J-I	0/1173	-28.0	-28.0	0.54 (2)	10.00														

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.8	PSF
DL	=	3.0	PSF	
BOT CH.	LL	=	10.5	PSF
DL	=	7.0	PSF	
TOTAL LOAD	=	49.1	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(65% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.81")
CALCULATED VERT. DEFL.(LL) = $L/899$ (0.23")
ALLOWABLE DEFL.(TL) = $L/360$ (0.81")
CALCULATED VERT. DEFL.(TL) = $L/761$ (0.36")

CS: TC=0.24/1.00 (B-C:1), BC=0.58/1.00 (J-L:2), WB=0.93/1.00 (C-M:1), SS=0.19/1.00 (L-M:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1687
	522	2284	1656

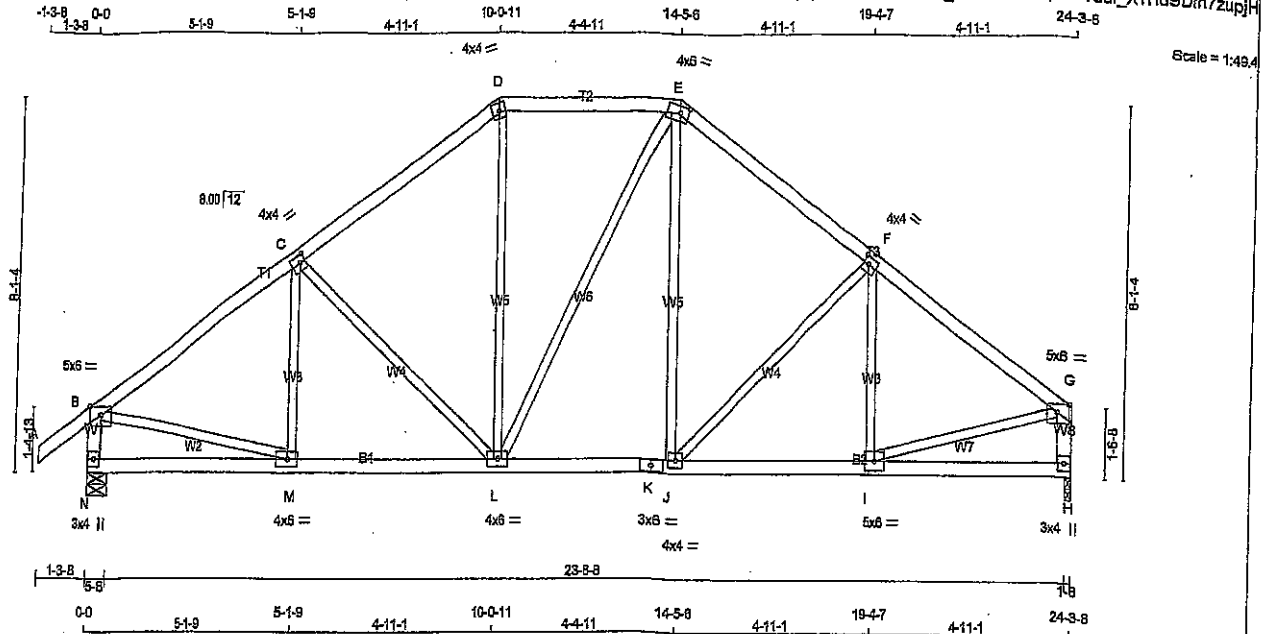
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.89 (M) (INPUT = 0.90)
JSI METAL = 0.40 (C) (INPUT = 1.00)



DRWG NO. TAM 3985-18
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
N - B	2x4	DRY	No.2
H - G	2x4	DRY	No.2
N - K	2x4	DRY	No.2
K - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (Table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TM/VW-p	MT20	5.0	6.0	Edge	
C TM/VW-t	MT20	4.0	4.0	2.00	1.50
D TTW-m	MT20	4.0	4.0		
E TTWV-m	MT20	4.0	8.0	1.75	2.50
F TM/VW-t	MT20	4.0	4.0	2.00	1.50
G TM/VW-p	MT20	5.0	6.0	Edge	
H BMV-t+p	MT20	3.0	4.0		
I BM/VW-t	MT20	5.0	6.0		
J BM/VW-t	MT20	4.0	4.0		
K BS-t	MT20	3.0	6.0		
L BM/VWV-t	MT20	4.0	6.0		
M BM/VW-t	MT20	4.0	6.0		
N BMV-t+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	RECORD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
N	1479	0	1479	0	5-8	5-8
H	1363	0	1363	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE MAX./MIN. COMPONENT REACTIONS						
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1198	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0
H	1119	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. FURLIN SPACING = 5.00 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 32	-84.3 -84.3	0.11 (1)	M-C	-115 / 147	0.04 (1)	
B-C	-1531 / 0	-84.3 -84.3	0.31 (1)	C-L	-354 / 0	0.30 (1)	
C-D	-1271 / 0	-84.3 -84.3	0.29 (1)	L-E	0 / 374	0.08 (2)	
D-E	-1037 / 0	-84.3 -84.3	0.22 (1)	E-F	0 / 15	0.00 (1)	
E-F	-1262 / 0	-84.3 -84.3	0.28 (1)	F-G	0 / 380	0.08 (2)	
F-G	-1483 / 0	-84.3 -84.3	0.29 (1)	G-H	-310 / 0	0.26 (1)	
N-B	-1420 / 0	0.0 0.0	0.15 (1)	H-I	-158 / 122	0.05 (1)	
H-G	-1307 / 0	0.0 0.0	0.14 (1)	I-G	0 / 1325	0.30 (1)	
					0 / 1284	0.29 (1)	
N-M	0 / 0	-28.0 -28.0	0.18 (3)				
M-L	0 / 1296	-28.0 -28.0	0.32 (2)				
L-K	0 / 1030	-28.0 -28.0	0.23 (2)				
K-J	0 / 1030	-28.0 -28.0	0.23 (2)				
J-I	0 / 1257	-28.0 -28.0	0.31 (2)				
I-H	0 / 0	-28.0 -28.0	0.17 (3)				

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 25.6 PSF
 DL = 3.0 PSF
 BOT CH. LL = 10.6 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, SOBC 2012, ABC 2014
 - CSA S8S-09
 - TPIC 2011

(65% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.81")
 CALCULATED VERT. DEFL.(LL) = L/689 (0.05")
 ALLOWABLE DEFL.(TL) = L/360 (0.81")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.08")

CSI: TC=0.31/1.00 (B-C:1), BC=0.32/1.00 (L-M:2), WB=0.30/1.00 (C-L:1), GS=0.17/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

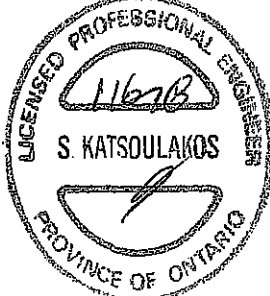
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION
 (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

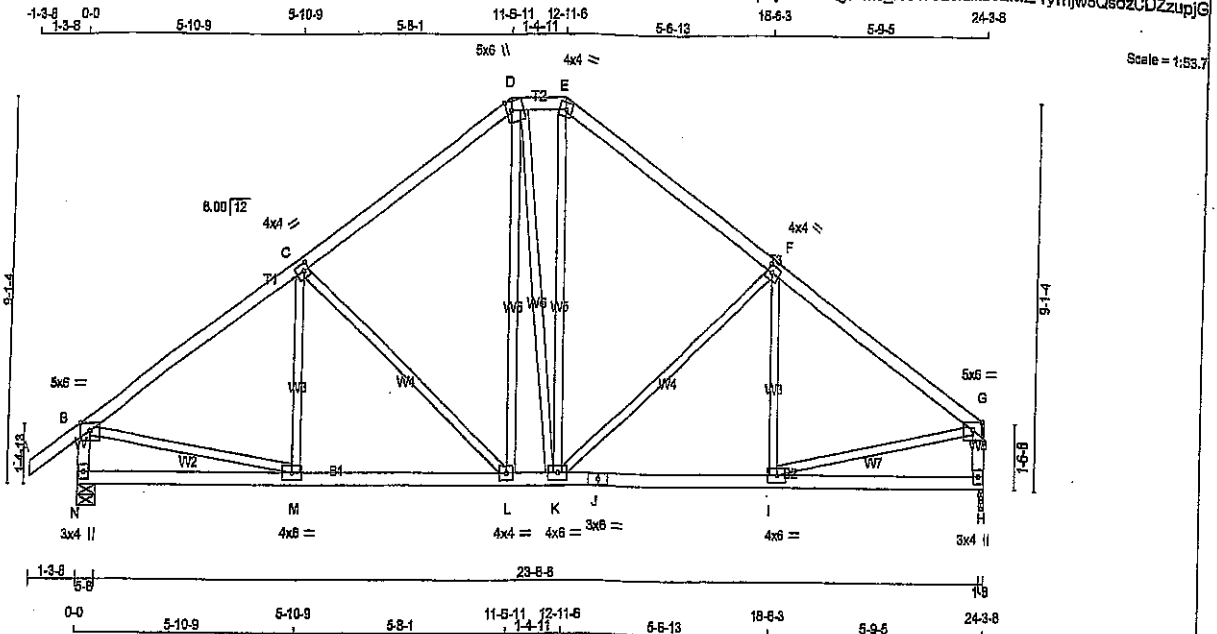
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (M) (INPUT = 0.90)
 JSI METAL= 0.32 (M) (INPUT = 1.00)



DRWG NO. TAM 3986-18
 STRUCTURAL
 COMPONENT ONLY

JOB NAME 285994	TRUSS NAME T7A	QUANTITY 1	PLY 1	JOB DESC. 43854	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.200 5 Dec 12 2017 Mitek Industries, Inc. Tue Jan 16 10:45:49 2018 Page 1	
				ID:W3K2IB2K4fGk9ID07SkJphylmC-HQgJem0_136475LelukB0aM21ymjw5QsozCDZzupjG	



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
N - B	2x4	DRY	No.2
H - G	2x4	DRY	No.2
N - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMVW-p	MT20	5.0	6.0 Edge
C	TMVW-t	MT20	4.0	4.0 2.00 1.50
D	TTVW+m	MT20	5.0	6.0 2.50 1.50
E	TTW-m	MT20	4.0	4.0
F	TMVW-t	MT20	4.0	4.0 2.00 1.50
G	TMVW-p	MT20	5.0	6.0 Edge
H	BMVW-p	MT20	3.0	4.0
I	BMVW-t	MT20	4.0	6.0
J	BS-t	MT20	3.0	6.0
K	BMVW-t	MT20	4.0	6.0
L	BMVW-t	MT20	4.0	4.0
M	BMVW-t	MT20	4.0	6.0
N	BMVW-p	MT20	3.0	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQD	
GROSS REACTION		GROSS REACTION		BRG		BRG	
JT	VERT	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX
N	1479	0	1479	0	5-8	5-8	5-8
H	1363	0	1363	0	1-6	1-8	1-8

UNFACTORED REACTIONS

1ST LOABE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1198	892/0	255/0	0/0	0/0	251/0	0/0
H	1119	521/0	255/0	0/0	0/0	243/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0/32	-84.3	-84.3 0.11 (1)	10.00	M-C	-50/214	0.05 (3)
B-C	-1533/0	-84.3	-84.3 0.42 (1)	4.86	C-L	-481/0	0.57 (1)
C-D	-1163/0	-84.3	-84.3 0.38 (1)	5.44	L-D	0/377	0.08 (1)
D-E	-945/0	-84.3	-84.3 0.03 (1)	6.25	D-K	0/17	0.00 (2)
E-F	-1164/0	-84.3	-84.3 0.37 (1)	5.46	K-E	0/390	0.09 (1)
F-G	-1491/0	-84.3	-84.3 0.39 (1)	4.94	K-F	-433/0	0.51 (1)
N-B	-1413/0	0.0	0.0 0.15 (1)	6.86	I-F	-84/185	0.04 (3)
H-G	-1299/0	0.0	0.0 0.14 (1)	7.08	B-M	0/1324	0.30 (1)
					I-G	0/1288	0.29 (1)
N-M	0/0	-28.0	-28.0 0.25 (3)	10.00			
M-L	0/1302	-28.0	-28.0 0.40 (2)	10.00			
L-K	0/943	-28.0	-28.0 0.25 (2)	10.00			
K-J	0/1288	-28.0	-28.0 0.37 (2)	10.00			
J-I	0/1288	-28.0	-28.0 0.37 (2)	10.00			
I-H	0/0	-28.0	-28.0 0.23 (3)	10.00			

TOTAL WEIGHT = 115 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL) = L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CSI: TC=0.42/1.00 (B-C:1), BC=0.40/1.00 (L-M:2), WB=0.57/1.00 (C-L:1), SS=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.60

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 522 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.33 (M) (INPUT = 1.00)



DRWG NO. TAM 3987-06
STRUCTURAL
COMPONENT ONLY



CONTINUED ON PAGE 2

JOB NAME 285301	TRUSS NAME T8Z2	QUANTITY 1	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:PFehc1Ba2cOYemTQTAYm8Oyl1EX-SjZyeeBsCc10ESJePyODQVUvYUmaAldDuZ67QazG tK

FACTORED CONCENTRATED LOADS (LBS)							
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
X	22-4-12	-40	-70	—	BACK	VERT	TOTAL
Y	23-10-12	-48	-85	—	BACK	VERT	TOTAL



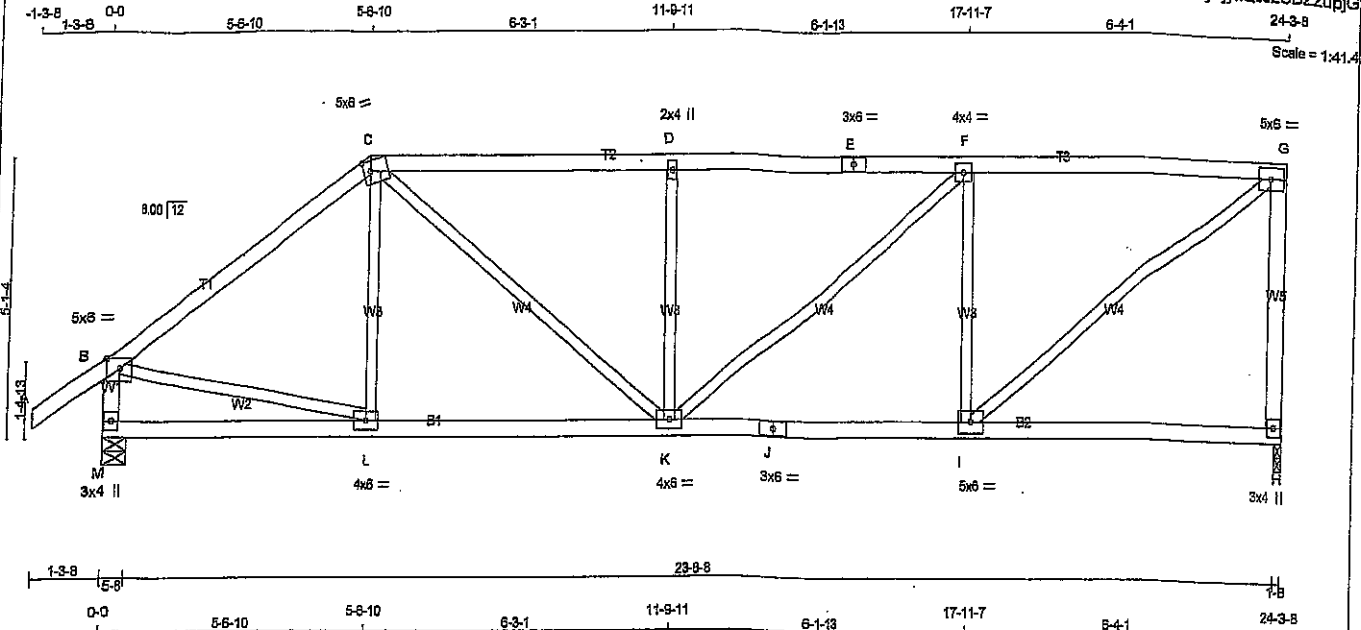
PM
 DWG NO. TAMZ6064-18
 STRUCTURAL
 COMPONENT ONLY

JOB NAME 285994	TRUSS NAME T9	QUANTITY 2	PLY 1	JOB DESC. 43954	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID: w3K2IB2K4fGk9ID07SkJphyfMC-HQgJem0_136475Leluk80aKe1y8jfrQsozCDZzupjG



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
H - G	2x4	DRY	No.2
M - B	2x4	DRY	No.2
M - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
DRY: SEASONED LUMBER.			

PLATES (table 1a in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMWV-p	MT20	5.0	6.0 Edge
C	TTWW-m	MT20	5.0	6.0 2.25 1.75
D	TMW-w	MT20	2.0	4.0
E	TS-t	MT20	3.0	6.0
F	TMWV-t	MT20	4.0	4.0
G	TMWV-t	MT20	5.0	6.0
H	BMV1-p	MT20	3.0	4.0
I	BMWV-t	MT20	5.0	6.0
J	BS-t	MT20	3.0	6.0
K	BMWV-t	MT20	4.0	6.0
L	BMWV-t	MT20	4.0	6.0
M	BMV1-p	MT20	3.0	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
H	1363	0	1363	0	0	1-8	1-8		
M	1479	0	1479	0	0	5-8	5-8		

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1119	821 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
M	1199	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINTS (H), (M)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.34 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM	TO		FR-TO		
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	L-C	-49 / 204 0.05 (3)
B-C	-1463 / 0	-84.3	-84.3	0.55 (1)	4.69	B-L	0 / 1263 0.28 (1)
C-D	-1787 / 0	-84.3	-84.3	0.55 (1)	4.37	I-G	0 / 1778 0.40 (1)
D-E	-1787 / 0	-84.3	-84.3	0.55 (1)	4.34	C-K	0 / 702 0.16 (1)
E-F	-1787 / 0	-84.3	-84.3	0.56 (1)	4.34	I-F	-877 / 0 0.34 (1)
F-G	-1416 / 0	-84.3	-84.3	0.53 (1)	4.81	K-D	-568 / 0 0.22 (1)
H-G	-1292 / 0	0.0	0.0	0.57 (1)	7.10	K-F	0 / 484 0.11 (1)
M-B	-1417 / 0	0.0	0.0	0.15 (1)	6.86		
M-L	0 / 0	-28.0	-28.0	0.24 (3)	10.00		
L-K	0 / 1240	-28.0	-28.0	0.39 (2)	10.00		
K-J	0 / 1416	-28.0	-28.0	0.44 (2)	10.00		
J-I	0 / 1416	-28.0	-28.0	0.44 (2)	10.00		
I-H	0 / 0	-28.0	-28.0	0.27 (3)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 3.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 49.1	PSF

SPACING = 240 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/899 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/999 (0.14")

CSI: TC=0.57/1.00 (G-H:1), BC=0.44/1.00 (H-K:2), WS=0.40/1.00 (G-I:1), SI=0.25/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.88 (K) (INPUT = 0.90)
JSI METAL= 0.44 (J) (INPUT = 1.00)

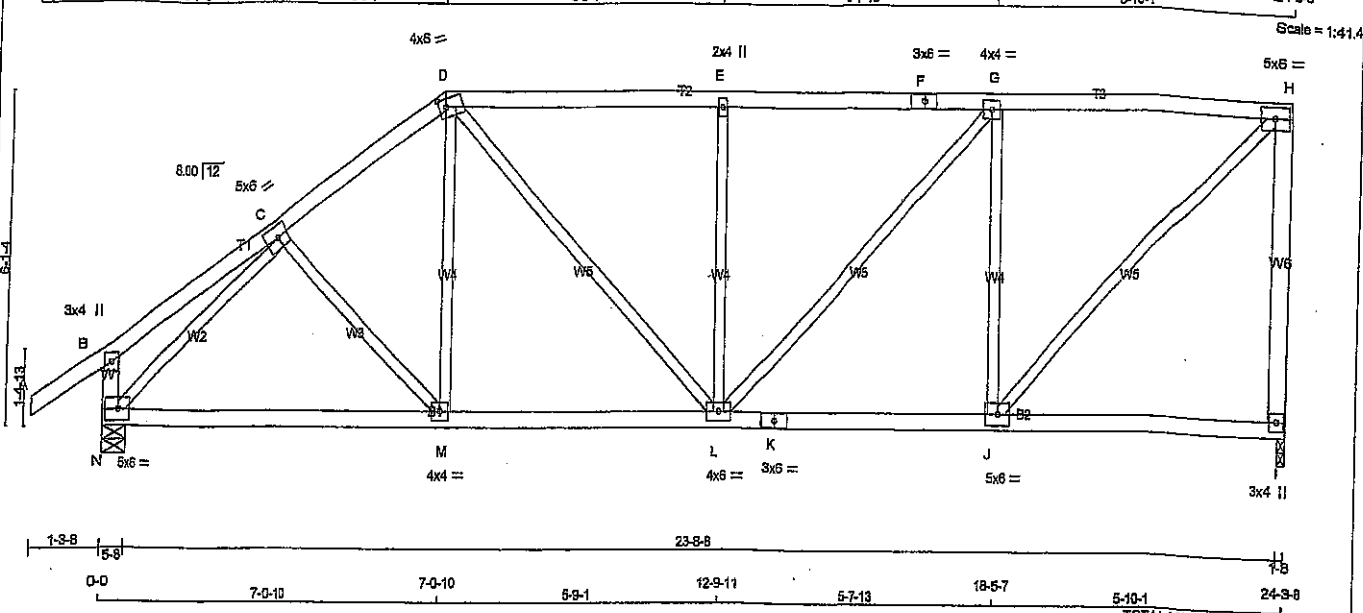


DWG NO. TAM 3988 -18
STRUCTURAL
COMPONENT ONLY

JOB NAME 285994	TRUSS NAME T10	QUANTITY 2	PLY 1	JOB DESC. 43954	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
I - H	2x4	DRY	No.2
N - B	2x4	DRY	No.2
N - K	2x4	DRY	No.2
K - I	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0	
C	TMWW-t	MT20	5.0	6.0	
D	TTWW-m	MT20	4.0	6.0	1.75
E	TMV+w	MT20	2.0	4.0	
F	TS-t	MT20	3.0	6.0	
G	TMWW-t	MT20	4.0	4.0	
H	TMV+p	MT20	5.0	6.0	
I	BMV1+p	MT20	3.0	4.0	
J	BMWW-t	MT20	5.0	6.0	
K	BS-t	MT20	3.0	6.0	
L	BMWW-t	MT20	4.0	6.0	
M	BMWW-t	MT20	4.0	4.0	
N	BMVW-t	MT20	5.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	DOWN	HORZ	UPLIFT
I	1363	0	1-8
N	1479	0	5-8

UNFACTORED REACTIONS

1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT COMBINED	1119	621/0	255/0	0/0	243/0	0/0
N	1198	692/0	255/0	0/0	251/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.96 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0/32	-84.3	-84.3 0.11 (1)	10.00	C-M	0/118	0.03 (3)
B-C	0/17	-84.3	-84.3 0.15 (1)	10.00	M-D	0/307	0.07 (2)
C-D	-1480/0	-84.3	-84.3 0.18 (1)	5.22	N-C	-1969/0	0.84 (1)
D-E	-1473/0	-84.3	-84.3 0.43 (1)	4.65	J-H	0/1557	0.35 (1)
E-F	-1473/0	-84.3	-84.3 0.44 (1)	4.66	D-L	0/385	0.08 (1)
F-G	-1473/0	-84.3	-84.3 0.44 (1)	4.66	J-G	-812/0	0.53 (1)
G-H	-1103/0	-84.3	-84.3 0.42 (1)	5.48	L-E	-520/0	0.30 (1)
I-H	-1296/0	0.0	0.0 0.94 (1)	7.08	L-G	0/530	0.12 (1)
N-B	-233/0	0.0	0.0 0.02 (1)	7.81			
N-M	0/1165	-28.0	-28.0 0.45 (2)	10.00			
M-L	0/1202	-28.0	-28.0 0.47 (2)	10.00			
L-K	0/1103	-28.0	-28.0 0.37 (2)	10.00			
K-J	0/1103	-28.0	-28.0 0.37 (2)	10.00			
J-I	0/0	-28.0	-28.0 0.24 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 3.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.61")

CALCULATED VERT. DEFL.(LL) = L/869 (0.11")

ALLOWABLE DEFL.(TL) = L/360 (0.61")

CALCULATED VERT. DEFL.(TL) = L/869 (0.19")

CSI: TC=0.94/1.00 (H-I), BC=0.47/1.00 (L-M-2), WB=0.94/1.00 (C-N-1), CSI=0.23/1.00 (G-H-1)

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

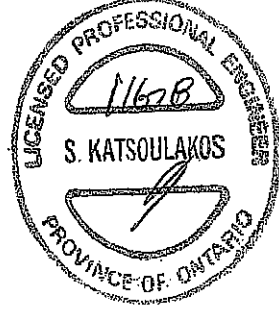
PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	PLI
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	818 354	1897 822	2284 1655

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.80 (D) (INPUT = 0.90)

JSI METAL= 0.41 (C) (INPUT = 1.00)



DWG NO. TAM 3989-13

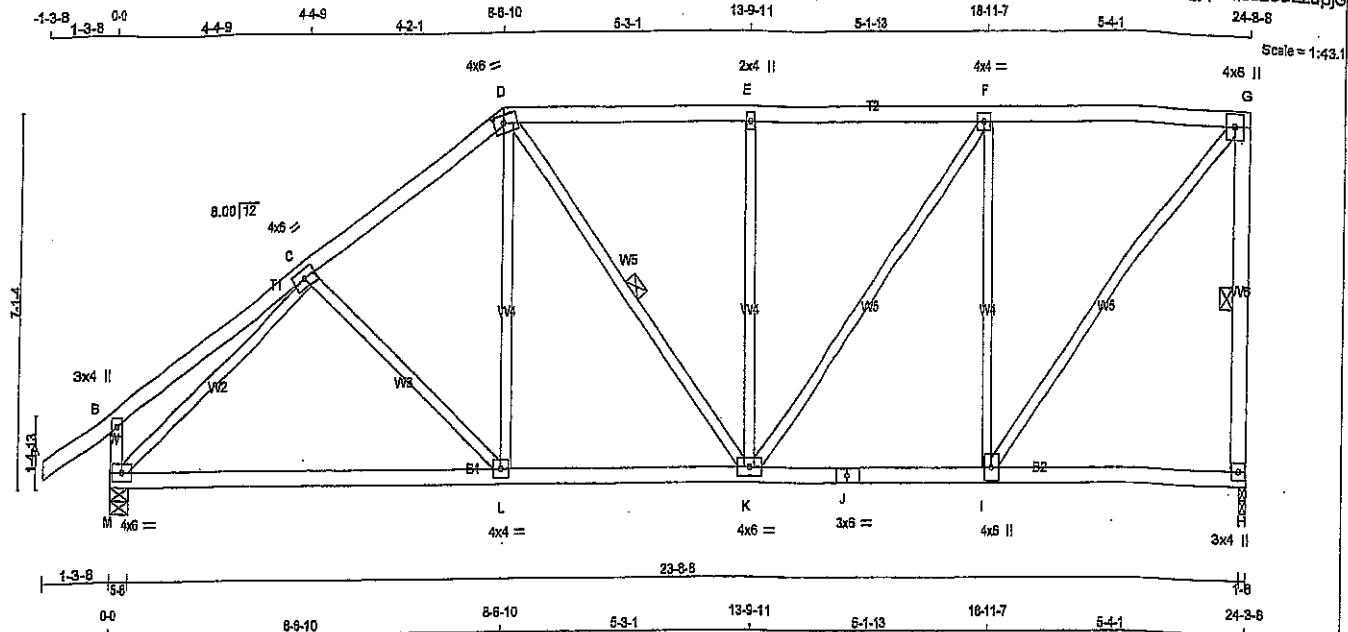
STRUCTURAL

COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
285994	T11	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 113 = 226 lb

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
H - G	2x4	DRY	No.2
M - B	2x4	DRY	No.2
M - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW-m	MT20	4.0	6.0	1.75	2.50
E	TMWW-w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMWW+p	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWW+t	MT20	4.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMWW-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	DOWN	GROSS REACTION	DOWN	BRG	IN-SX	BRG	IN-SX
H	1363	0	1363	0	0	1-8	1-8	1-8	1-8
M	1479	0	1479	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

UNIT FACTORED REACTION							
1ST CASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
H	1119	621/0	255/0	0/0	0/0	243/0	0/0
M	1198	692/0	255/0	0/0	0/0	251/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-H, D-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	MAX. FACTORED CSI (LC)	
FR-TO		FROM	TO		FR-TO				
A-B	0/32	-84.3	-84.3	0.11 (1)	10.00	C-L	-117/85	0.07 (1)	
B-C	0/23	-84.3	-84.3	0.24 (1)	10.00	L-D	0/414	0.09 (2)	
C-D	-1390/0	-84.3	-84.3	0.28 (1)	5.20	M-C	-1681/0	0.83 (1)	
D-E	-1228/0	-84.3	-84.3	0.34 (1)	5.37	I-G	0/1426	0.32 (1)	
E-F	-1227/0	-84.3	-84.3	0.35 (1)	5.35	D-K	-6/141	0.03 (1)	
F-G	-679/0	-84.3	-84.3	0.34 (1)	6.07	I-F	-946/0	0.83 (1)	
H-G	-1805/0	0.0	0.0	0.30 (1)	5.88	K-E	-474/0	0.41 (1)	
M-B	-255/0	0.0	0.0	0.08 (1)	7.81	K-F	0/578	0.13 (1)	
M-L	0/1223	-28.0	-28.0	0.58 (2)	10.00				
L-K	0/1141	-28.0	-28.0	0.59 (2)	10.00				
K-J	0/879	-28.0	-28.0	0.31 (2)	10.00				
J-I	0/879	-28.0	-28.0	0.31 (2)	10.00				
I-H	0/0	-28.0	-28.0	0.21 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/999 (0.23")
ALLOWABLE DEFL.(TL) = L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/779 (0.37")

CS: TC=0.35/1.00 (E-F-1), BC=0.59/1.00 (K-L-2), WB=0.93/1.00 (C-M-1), SS=0.21/1.00 (F-G-1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

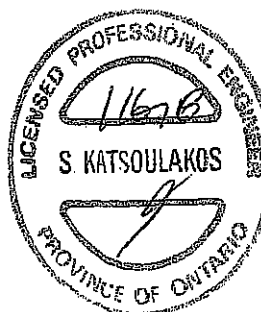
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2264 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.90 (M) (INPUT = 0.90)
JSI METAL= 0.40 (C) (INPUT = 1.00)



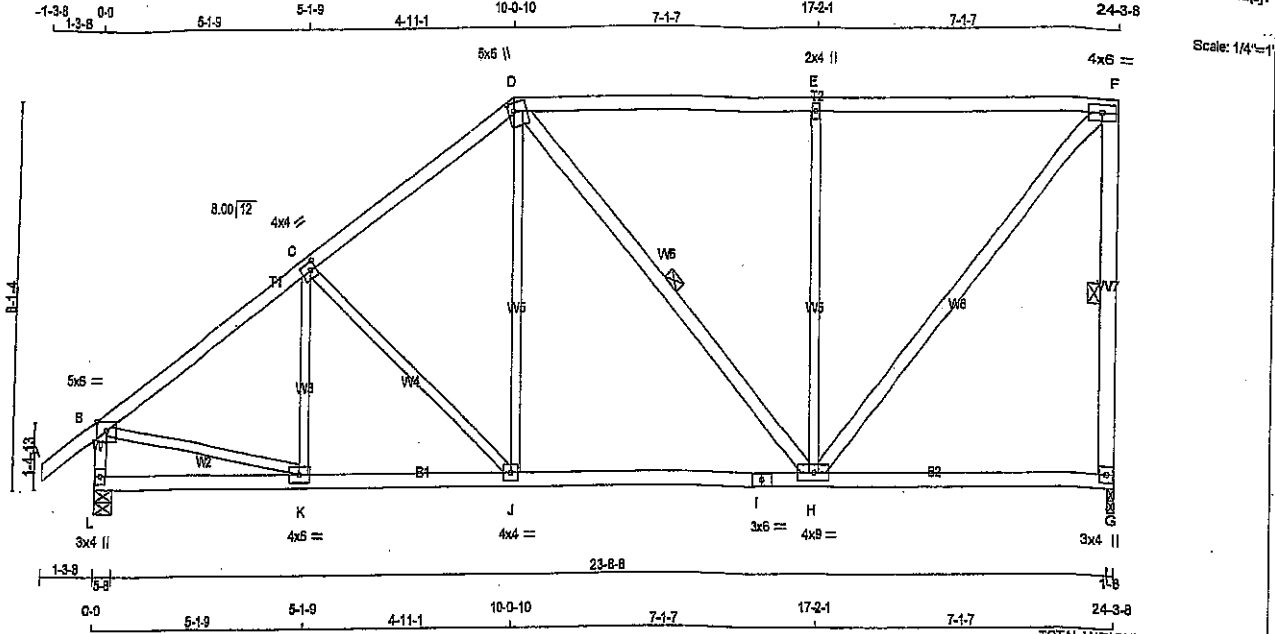
DRWG NO. TAM 3990-18
STRUCTURAL
COMPONENT ONLY

JOB NAME 285994	TRUSS NAME T12	QUANTITY 2	PLY 1	JOB DESC. TRUSS DESC.	43954	DRWG NO.
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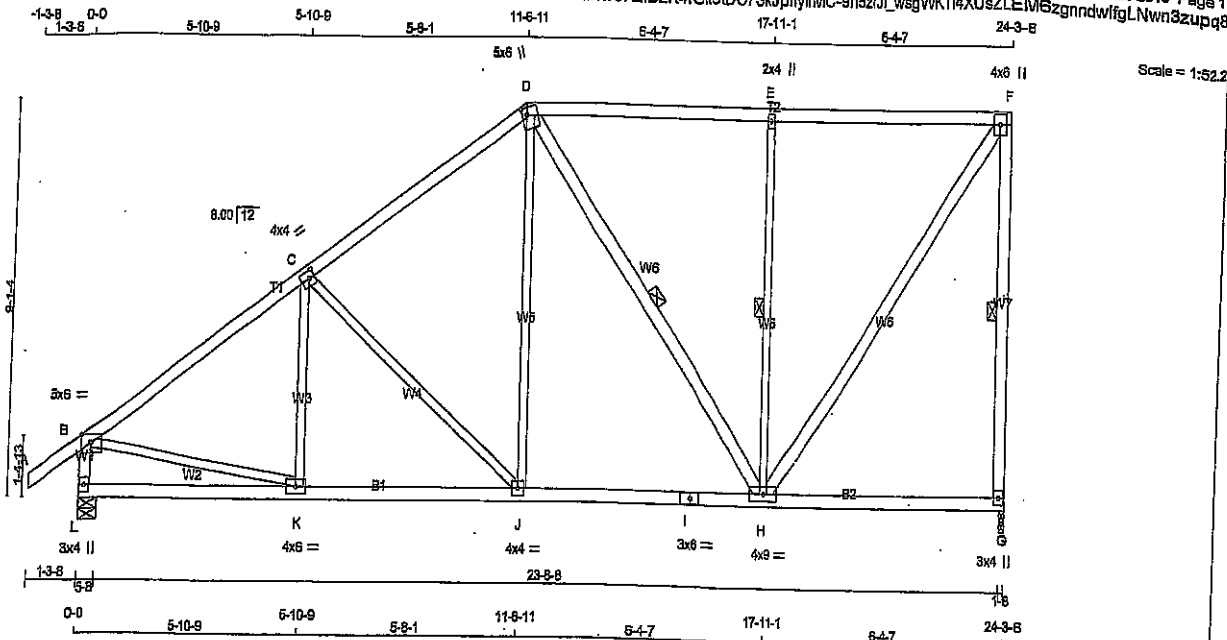
Tamarack Roof Truss, Burlington

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JOB NAME 286039	TRUSS NAME T13	QUANTITY 5	PLY 1	JOB DESC. 43954	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.200 S Dec 12 2017 Mitek Industries, Inc. Tue Jan 16 10:38:28 2018 Page 1	



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
G - F	2x4	DRY	No.2
L - B	2x4	DRY	No.2
L - I	2x4	DRY	No.2
I - G	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
D - H	2x4	DRY	No.2
H - F	2x4	DRY	No.2

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
G	1363	0	1363	0
L	1479	0	1479	0

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
G	1119	821 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
L	1198	892 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.61 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO				FR-TO			
A-B	0 / 32	-84.3	-84.3 0.11 (1)	K-C	-58 / 203	0.05 (3)	
B-C	-1531 / 0	-84.3	-84.3 0.59 (1)	C-J	-474 / 0	0.56 (1)	
C-D	-1167 / 0	-84.3	-84.3 0.54 (1)	J-O	0 / 490	0.11 (2)	
D-E	-790 / 0	-84.3	-84.3 0.61 (1)	D-H	-270 / 0	0.17 (1)	
E-F	-780 / 0	-84.3	-84.3 0.62 (1)	H-E	-866 / 0	0.38 (1)	
G-F	-1292 / 0	0.0	0.0 0.50 (1)	H-F	0 / 1344	0.22 (1)	
L-B	-1412 / 0	0.0	0.0 0.15 (1)	B-K	0 / 1322	0.30 (1)	
L-K	0 / 0	-28.0	-28.0 0.23 (3)				
K-J	0 / 1300	-28.0	-28.0 0.56 (2)				
J-I	0 / 948	-28.0	-28.0 0.40 (2)				
I-H	0 / 948	-28.0	-28.0 0.40 (2)				
H-G	0 / 0	-28.0	-28.0 0.29 (3)				

PLATES (table ts in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TTVW+m	MT20	5.0	6.0	2.00	1.50
E	TMVW-w	MT20	2.0	4.0		
F	TMVW-p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMVW-w	MT20	4.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMVW-t	MT20	4.0	4.0		
K	BMVW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
DL =	3.0	PSF	
BOT CH.	LL =	10.5	PSF
DL =	7.0	PSF	
TOTAL LOAD =	46.1	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = 1/360 (0.81")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.07")
ALLOWABLE DEFL.(TL) = 1/360 (0.81")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.11")

CSI: TC=0.62/1.00 (E-F:1), BC=0.40/1.00 (H-J:2), WB=0.55/1.00 (C-J:1), SS=0.26/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.90 (H) (INPUT = 0.90)

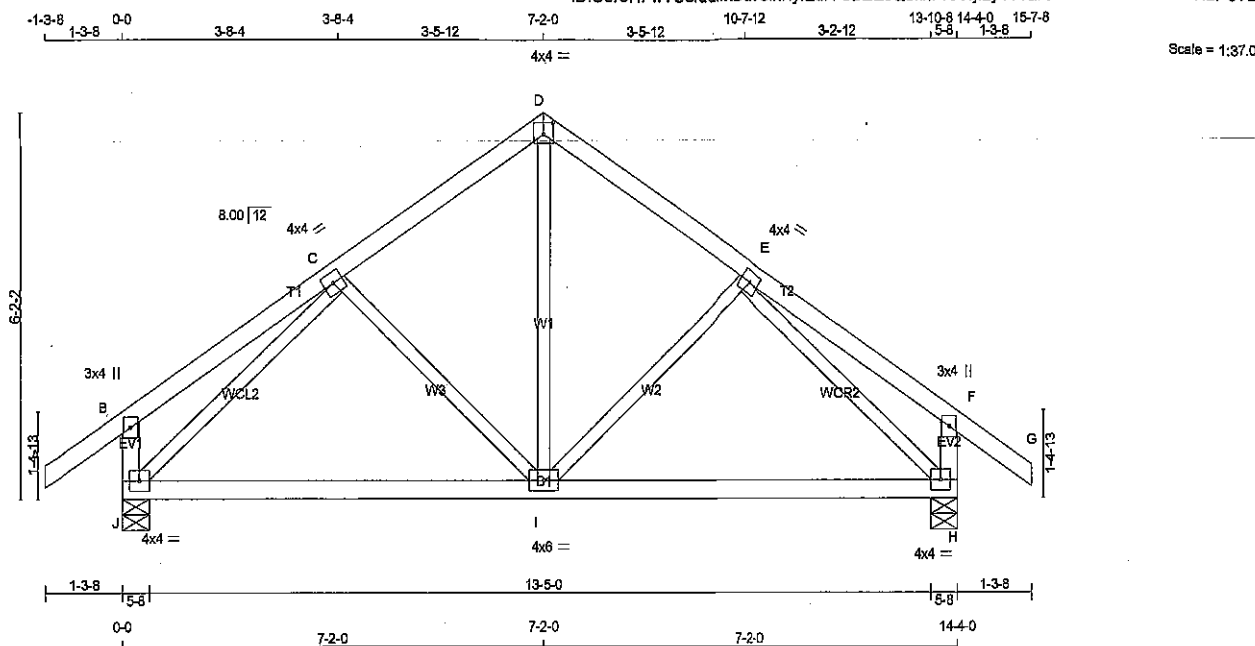
JSI METAL = 0.32 (K) (INPUT = 1.00)



DRWG NO. TAM 3993-12
STRUCTURAL
COMPONENT ONLY

JOB NAME 285994	TRUSS NAME T14	QUANTITY 4	PLY 1	JOB DESC. 43954	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.200 S Jan 6 2018 M/Tek Industries, Inc. Thu Apr 19 13:43:04 2018 Page 1
ID:Uer6HFwTUstufdnkBu50lkHylEuR-eBEEsQ3khFRA4jI2y4V7LrOdPwnNSA30vnC87IzP97L



TOTAL WEIGHT = 4 X 62 = 247 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	DRY
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
J - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)			
JT	TYPE	PLATES	W LEN Y X
B	TMV+p	MT20	3.0 4.0
C	TMWW-t	MT20	4.0 4.0
D	TTW-p	MT20	4.0 4.0 2.25 2.00
E	TMWW-t	MT20	4.0 4.0
F	TMV+p	MT20	3.0 4.0
H	BMVW1-t	MT20	4.0 4.0
I	BMVWW-t	MT20	4.0 6.0
J	BMVW1-t	MT20	4.0 4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
J	920	0	920	0	0	5-8	5-8	5-8	5-8
H	920	0	920	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
J	739	437 / 0	151 / 0	0 / 0	0 / 0	152 / 0	0 / 0	
H	739	437 / 0	151 / 0	0 / 0	0 / 0	152 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 32	-84.3 -84.3	0.11 (1)	I-D	0 / 460	0.10 (1)	
B-C	0 / 19	-84.3 -84.3	0.17 (1)	I-E	-152 / 44	0.08 (1)	
C-D	-645 / 0	-84.3 -84.3	0.13 (1)	C-I	-152 / 44	0.08 (1)	
D-E	-645 / 0	-84.3 -84.3	0.13 (1)	J-C	-879 / 0	0.35 (1)	
E-F	0 / 19	-84.3 -84.3	0.17 (1)	E-H	-879 / 0	0.35 (1)	
F-G	0 / 32	-84.3 -84.3	0.11 (1)				
J-B	-232 / 0	0.0 0.0	0.02 (1)				
H-F	-232 / 0	0.0 0.0	0.02 (1)				
J-I	0 / 631	-28.0 -28.0	0.49 (2)				
I-H	0 / 631	-28.0 -28.0	0.49 (2)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.48")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (0.48")
CALCULATED VERT. DEFL.(TL) = L/999 (0.13")

CSI: TC=0.17/1.00 (E-F:1), BC=0.49/1.00 (H-I:2), WB=0.35/1.00 (E-H:1), SS=0.17/1.00 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

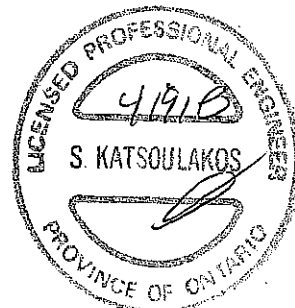
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

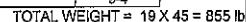
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (D) (INPUT = 0.90)
JSI METAL= 0.31 (C) (INPUT = 1.00)



DWG NO. TAM 2125-18
STRUCTURAL
COMPONENT ONLY

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ID:w3K2IB2K4fGk9tD07Sk|phvIFMC-yHxHsz4PfoPlTM BevWxvoNT0?6mXnK?Kf82cyfe8J



Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)		
FR-TO		FROM TO			FR-TO				
A-B	-4/0	-84.3	-84.3	0.02 (1)	10.00	F-C	-420/0 0.06 (1)		
B-I	-1258/0	-84.3	-84.3	0.03 (1)	6.25	F-D	0/1157 0.26 (1)		
I-C	-1158/0	-84.3	-84.3	0.07 (1)	6.25	H-I	0/131 0.00 (1)		
C-D	-1152/0	-84.3	-84.3	0.17 (1)	6.25	D-J	-785/0 0.03 (1)		
E-G	0/106	0.0	0.0	0.34 (1)	10.00	G-J	0/558 0.00 (1)		
G-D	0/106	0.0	0.0	0.34 (1)	10.00				
B-H	0/1128	-28.0	-28.0	0.20 (1)	10.00				
H-F	0/1128	-28.0	-28.0	0.28 (2)	10.00				
F-E	0/103	-28.0	-28.0	0.17 (2)	10.00				

JSI GRIP= 0.87 (F) (INPUT = 0.90)
JSI METAL= 0.40 (B) (INPUT = 1.00)



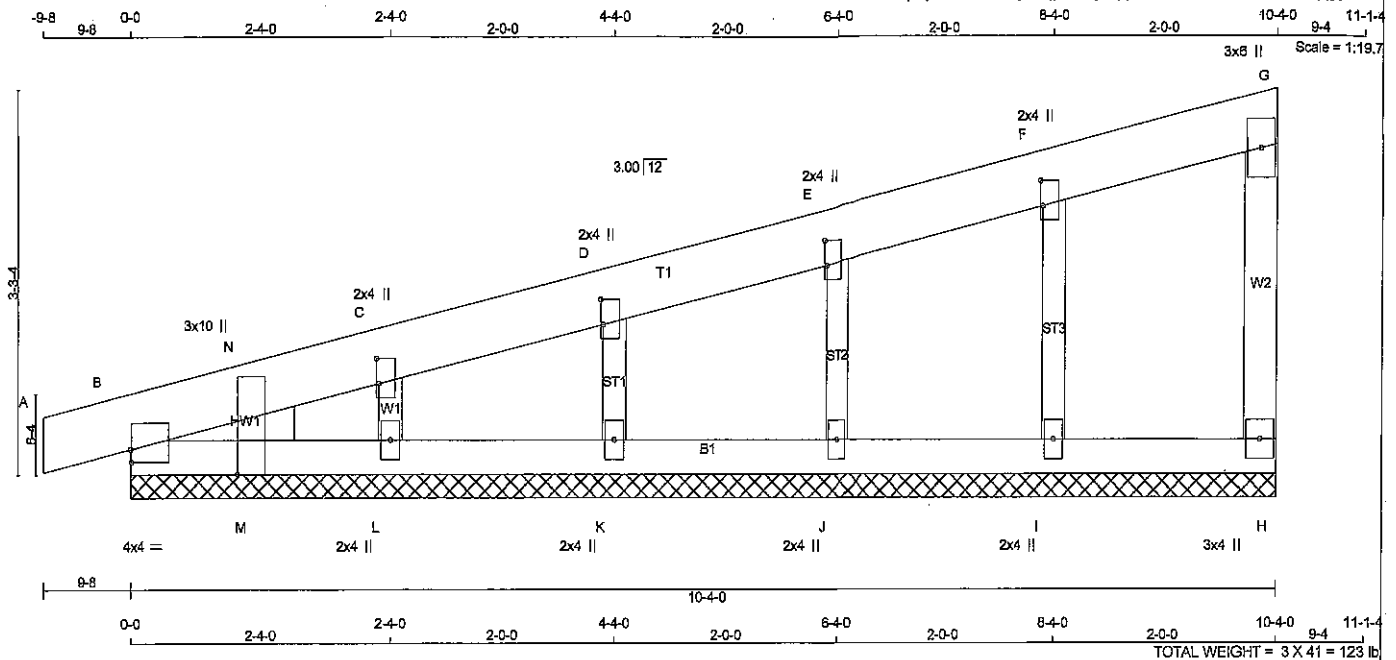
DWONG, TAM 45631-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
285993	G90	3	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:w3K2IB2K4fGk9tD07SkJphylfMC-77Of1pvJqp2GayuqqNr6heYQub0aMtuRcPCqqyfe8P



LUMBER						
N. L. G. A. RULES						
CHORDS		SIZE		LUMBER	DESCR.	
A - G		2x6	DRY	No.2		SPF
H - G		2x4	DRY	No.2		SPF
B - H		2x4	DRY	No.2		SPF
ALL WEBS		2x3	DRY	No.2		SPF
ALL GABLE WEBS						
		2x3	DRY	No.2		SPF
DRY: SEASONED LUMBER.						

GABLE STUDS SPACED AT 2'-0" O.C.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	4.0	4.0	1.25	
B	WP-I	MT20	3.0	10.0	Edge	11.50
C, D, E, F						
C	TMW+w	MT20	2.0	4.0	2.50	0.25
G	TMV+p	MT20	3.0	8.0		
H	BMV1+p	MT20	3.0	4.0		
I, J, K, L						
I	BMW1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

HEEL

WEDGE

2x4 L

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)	UNBRACED LENGTH (FT)
FR-TO				FR-TO			
A-B	-41/0	-84.3	-84.3 0.02 (1)	10.00	I-F	-188/0	0.03 (1)
B-N	-48/0	-84.3	-84.3 0.02 (1)	8.25	J-E	-168/0	0.03 (1)
N-C	-14/3	-84.3	-84.3 0.03 (1)	6.25	K-D	-158/0	0.02 (1)
C-D	-18/0	-84.3	-84.3 0.03 (1)	8.25	L-C	-201/0	0.03 (1)
D-E	-9/0	-84.3	-84.3 0.02 (1)	10.00	M-N	0/41	0.00 (1)
E-F	-4/0	-84.3	-84.3 0.02 (1)	10.00			
F-G	-4/0	-84.3	-84.3 0.02 (1)	10.00			
H-G	-72/0	0.0	0.0 0.01 (1)	7.81			
B-M	0/28	-28.0	-28.0 0.02 (1)	10.00			
M-L	0/28	-28.0	-28.0 0.03 (2)	10.00			
L-K	0/16	-28.0	-28.0 0.02 (2)	10.00			
K-J	0/9	-28.0	-28.0 0.02 (2)	10.00			
J-I	0/4	-28.0	-28.0 0.03 (2)	10.00			
I-H	0/0	-28.0	-28.0 0.03 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
	DL = 3.0 PSF
BOT CH.	LL = 10.5 PSF
	DL = 7.0 PSF
TOTAL LOAD	= 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.03 (C-D:1), BC=0.03 (I-J:2), WB=0.03 (F-I:1), SSI=0.07 (C-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354 1667 822 2284 1858

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (C) (INPUT = 0.90)
JSI METAL= 0.04 (L) (INPUT = 1.00)



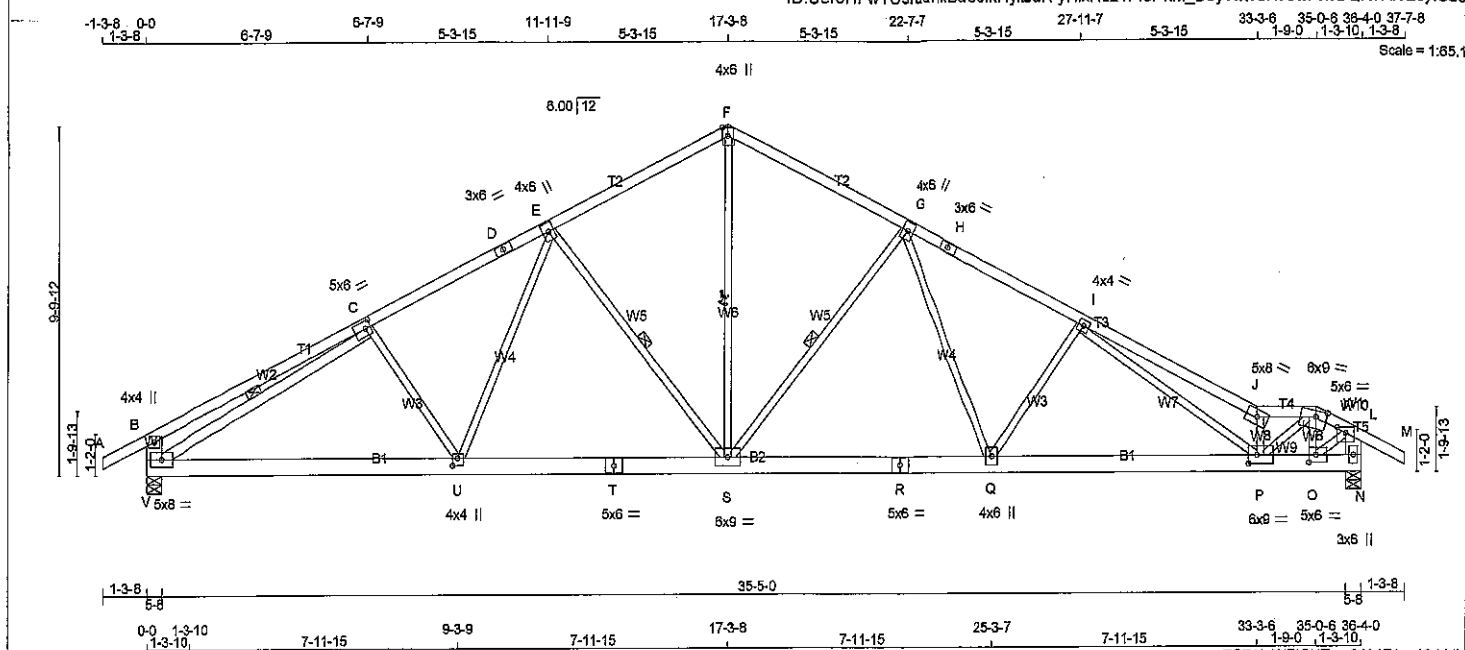
DWG NO. TAM 4566817
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
285993	T105	6	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:Uer6HFwTUfudnkBu50lkHylEuR-yHlxHsz4PfoPHM_BeyWxvoKv0wFmOQK?Kf82cyfe8J



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - H	2x4	DRY	No.2	SPF	
H - J	2x4	DRY	No.2	SPF	
J - K	2x4	DRY	No.2	SPF	
K - M	2x4	DRY	No.2	SPF	
V - B	2x6	DRY	No.2	SPF	
N - L	2x6	DRY	No.2	SPF	
V - T	2x6	DRY	No.2	SPF	
T - R	2x6	DRY	No.2	SPF	
R - N	2x6	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
V - C	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMVW-t	MT20	5.0	6.0	2.50	2.00
D	TS-t	MT20	3.0	6.0		
E	TMVW+t	MT20	4.0	6.0		
F	TTW+p	MT20	4.0	6.0	Edge	
G	TMVW+t	MT20	4.0	6.0		
H	TS-t	MT20	3.0	6.0		
I	TMVW-t	MT20	4.0	4.0		
J	TTW-h	MT20	5.0	6.0		
K	TTWW-m	MT20	6.0	9.0	2.00	3.50
L	TMVW-p	MT20	5.0	6.0	2.00	3.00
N	BMV1+p	MT20	3.0	6.0		
O	BMVW-t	MT20	5.0	6.0	2.50	2.50
P	BMVWW-t	MT20	6.0	9.0	3.00	3.00
Q	BMVW+t	MT20	4.0	6.0		
R	BS-t	MT20	5.0	6.0		
S	BMVWW-t	MT20	6.0	9.0		
T	BS-t	MT20	5.0	6.0		
U	BMVW+t	MT20	4.0	4.0	2.50	1.75
V	BMVW-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	UPLIFT
N	2156	0	2156	0
V	2151	0	2151	0

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1753	1000 / 0	382 / 0	0 / 0	0 / 0	372 / 0	0 / 0
V	1751	998 / 0	382 / 0	0 / 0	0 / 0	371 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, V

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.08 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-S, G-S, C-V. *F-51*

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-84.3 -84.3	0.11 (1)	C-U	-220 / 88	0.09 (1)	
B-C	0 / 21	-84.3 -84.3	0.43 (1)	U-E	0 / 509	0.11 (2)	
C-D	-2936 / 0	-84.3 -84.3	0.49 (1)	E-S	-763 / 0	0.38 (1)	
D-E	-2936 / 0	-84.3 -84.3	0.49 (1)	S-F	0 / 1603	0.36 (1)	
E-F	-2220 / 0	-84.3 -84.3	0.36 (1)	F-G	-1019 / 0	0.51 (1)	
F-G	-2222 / 0	-84.3 -84.3	0.36 (1)	G-Q	0 / 826	0.19 (1)	
G-H	-3245 / 0	-84.3 -84.3	0.44 (1)	Q-I	-578 / 0	0.23 (1)	
H-I	-3245 / 0	-84.3 -84.3	0.44 (1)	I-P	0 / 686	0.15 (1)	
I-J	-4201 / 0	-84.3 -84.3	0.57 (1)	P-K	-2138 / 0	0.31 (1)	
J-K	-3623 / 0	-84.3 -84.3	0.17 (1)	K-J	0 / 2401	0.54 (1)	
K-L	-2116 / 0	-84.3 -84.3	0.13 (1)	O-K	-704 / 0	0.10 (1)	
L-M	0 / 26	-84.3 -84.3	0.11 (1)	O-L	0 / 2127	0.48 (1)	
V-B	-341 / 0	0.0 0.0	0.02 (1)	V-C	-3227 / 0	0.86 (1)	
N-L	-2199 / 0	0.0 0.0	0.14 (1)				
V-U	0 / 2744	-28.0 -28.0	0.48 (2)				
U-T	0 / 2443	-28.0 -28.0	0.45 (2)				
T-S	0 / 2443	-28.0 -28.0	0.45 (2)				
S-R	0 / 2602	-28.0 -28.0	0.45 (1)				
R-Q	0 / 2602	-28.0 -28.0	0.45 (1)				
Q-P	0 / 3230	-28.0 -28.0	0.59 (1)				
P-O	0 / 1812	-28.0 -28.0	0.38 (1)				
O-N	0 / 0	-28.0 -28.0	0.06 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.8	PSF
	DL = 3.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 48.1	PSF

SPACING = 24.0 IN./C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (1.21")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.22")
ALLOWABLE DEFL.(TL) = $L/360$ (1.21")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.36")

CSI: TC=0.57 (L-U), BC=0.59 (P-Q), WB=0.86 (C-V), SSI=0.23 (B-C)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

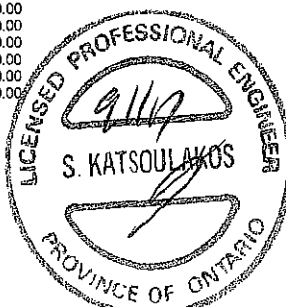
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1658

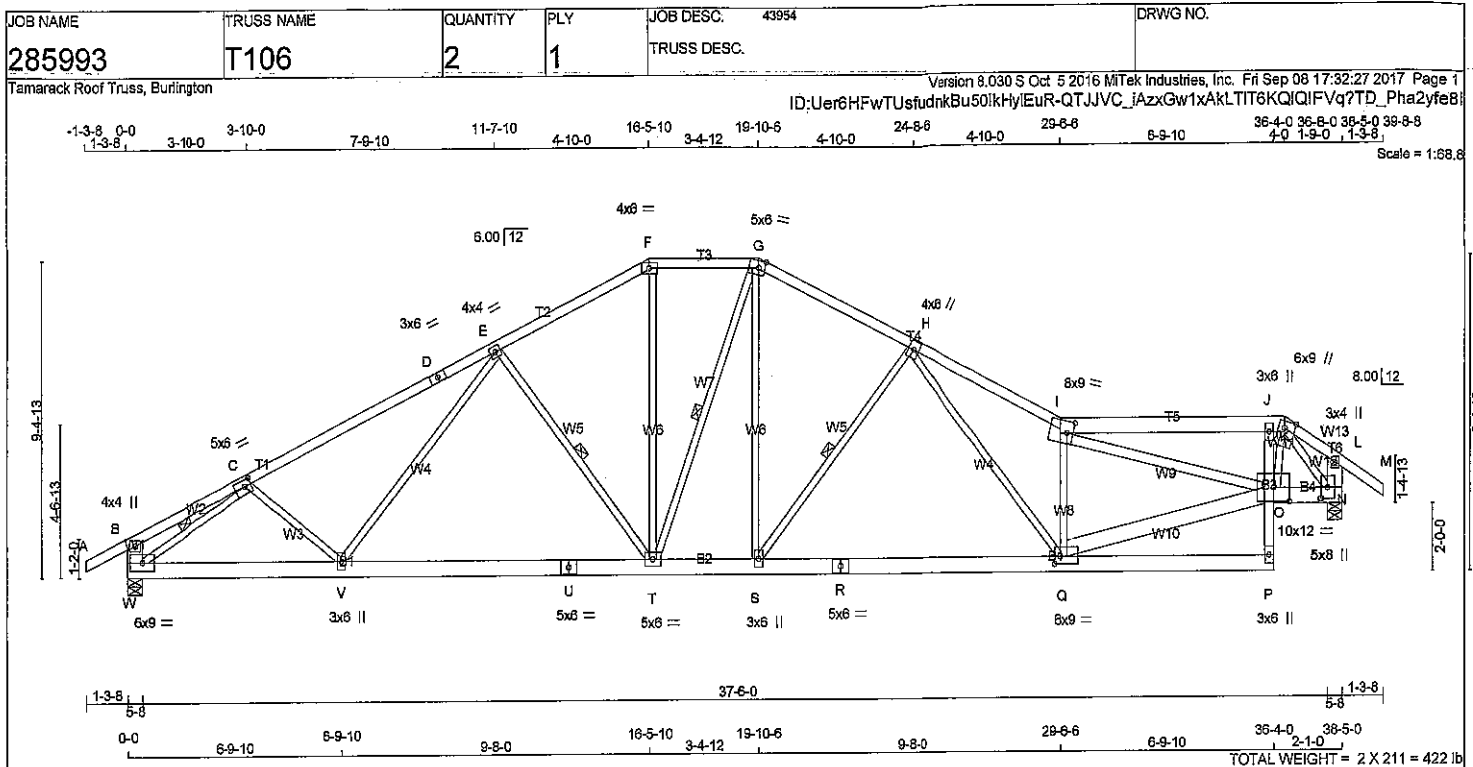
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.89 (U) (INPUT = 0.80)
JSI METAL= 0.79 (C) (INPUT = 1.00)

DRW NO. TAM 45633-17
STRUCTURAL
COMPONENT ONLY





LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - K	2x6	DRY	No.2
K - M	2x4	DRY	No.2
W - B	2x6	DRY	No.2
N - L	2x6	DRY	No.2
W - U	2x6	DRY	No.2
U - R	2x6	DRY	No.2
R - P	2x6	DRY	No.2
P - J	2x4	DRY	No.2
O - N	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
Q - O	2x6	DRY	No.2
I - O	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	4.0	4.0
C	TMVW+I	MT20	5.0	8.0 2.25 2.50
D	TS-I	MT20	3.0	6.0
E	TMVW+I	MT20	4.0	4.0
F	TTW-I	MT20	4.0	6.0
G	TTW+I	MT20	5.0	6.0 2.50 2.25
H	TMVW+I	MT20	4.0	6.0
I	TTW+I	MT20	8.0	9.0 4.25 2.25
J	TMV+p	MT20	3.0	6.0
K	TTW+I	MT20	8.0	9.0 Edge 3.75
L	TMV+p	MT20	3.0	4.0
N	BMVW+I	MT20	5.0	8.0 4.00 2.25
O	BMVW+I	MT20	10.0	12.0 5.00 5.75
P	BMV+p	MT20	3.0	6.0
Q	BMVW+I	MT20	6.0	9.0 2.75 2.25
R	BS-I	MT20	5.0	6.0
S	BMVW+I	MT20	3.0	6.0
T	BMVW+I	MT20	5.0	6.0
U	BS-I	MT20	5.0	6.0
V	BMVW+I	MT20	3.0	6.0
W	BMVW+I	MT20	8.0	9.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	GROSS REACTION	BRG	BRG
W	2268 0	2268 0	0 0	5-8	5-8
N	2274 0	2274 0	0 0	5-8	5-8

UNFACTORED REACTIONS							
1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
W	1847	1051 / 0	403 / 0	0 / 0	0 / 0	392 / 0	0 / 0
N	1850	1054 / 0	403 / 0	0 / 0	0 / 0	393 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.17 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-T, G-T, H-S, C-W.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FORCE (LBS)
FR-TO		FROM TO		FR-TO	
A-B	0 / 26	-84.3 -84.3	0.11 (1)	C-V	0 / 227
B-C	0 / 39	-84.3 -84.3	0.45 (1)	V-E	0 / 400
C-D	-3180 / 0	-84.3 -84.3	0.88 (1)	E-T	-701 / 0
D-E	-3180 / 0	-84.3 -84.3	0.88 (1)	T-F	0 / 887
E-F	-2585 / 0	-84.3 -84.3	0.51 (1)	F-G	-105 / 0
F-G	-2271 / 0	-84.3 -84.3	0.18 (1)	G-H	0 / 954
G-H	-2591 / 0	-84.3 -84.3	0.36 (1)	H-I	-981 / 0
H-I	-3987 / 0	-84.3 -84.3	0.50 (1)	I-J	0 / 1166
I-J	-2181 / 0	-84.3 -84.3	0.28 (1)	J-K	-1762 / 0
J-K	-2229 / 0	-84.3 -84.3	0.12 (1)	K-L	0 / 3684
K-L	-25 / 0	-84.3 -84.3	0.08 (1)	L-M	-1450 / 0
L-M	0 / 32	-84.3 -84.3	0.11 (1)	M-N	-3472 / 0
M-N	-185 / 0	0.0 0.0	0.01 (1)	N-O	0 / 2551
N-O	-235 / 0	0.0 0.0	0.02 (1)	O-P	-2596 / 0

W-V	0 / 2835	-28.0 -28.0	0.44 (2)	10.00
V-U	0 / 2881	-28.0 -28.0	0.45 (1)	10.00
U-T	0 / 2581	-28.0 -28.0	0.45 (1)	10.00
T-S	0 / 2307	-28.0 -28.0	0.41 (2)	10.00
S-R	0 / 2884	-28.0 -28.0	0.47 (1)	10.00
R-Q	0 / 2884	-28.0 -28.0	0.47 (1)	10.00
Q-P	0 / 24	-28.0 -28.0	0.22 (3)	10.00
P-O	0 / 116	0.0 0.0	0.08 (1)	10.00
O-J	-870 / 0	0.0 0.0	0.08 (1)	10.00
J-I	0 / 1645	-28.0 -28.0	0.21 (1)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN./C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.28")
CALCULATED VERT. DEFL.(LL) = L/999 (0.24")
ALLOWABLE DEFL.(TL) = L/360 (1.28")
CALCULATED VERT. DEFL.(TL) = L/999 (0.40")

CSI: TC=0.88 (C-E:1), BC=0.47 (Q-S:1), WB=0.90 (I-O:1), SSI=0.26 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

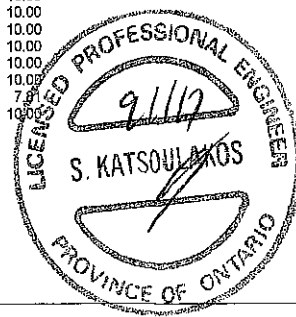
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2264 1656

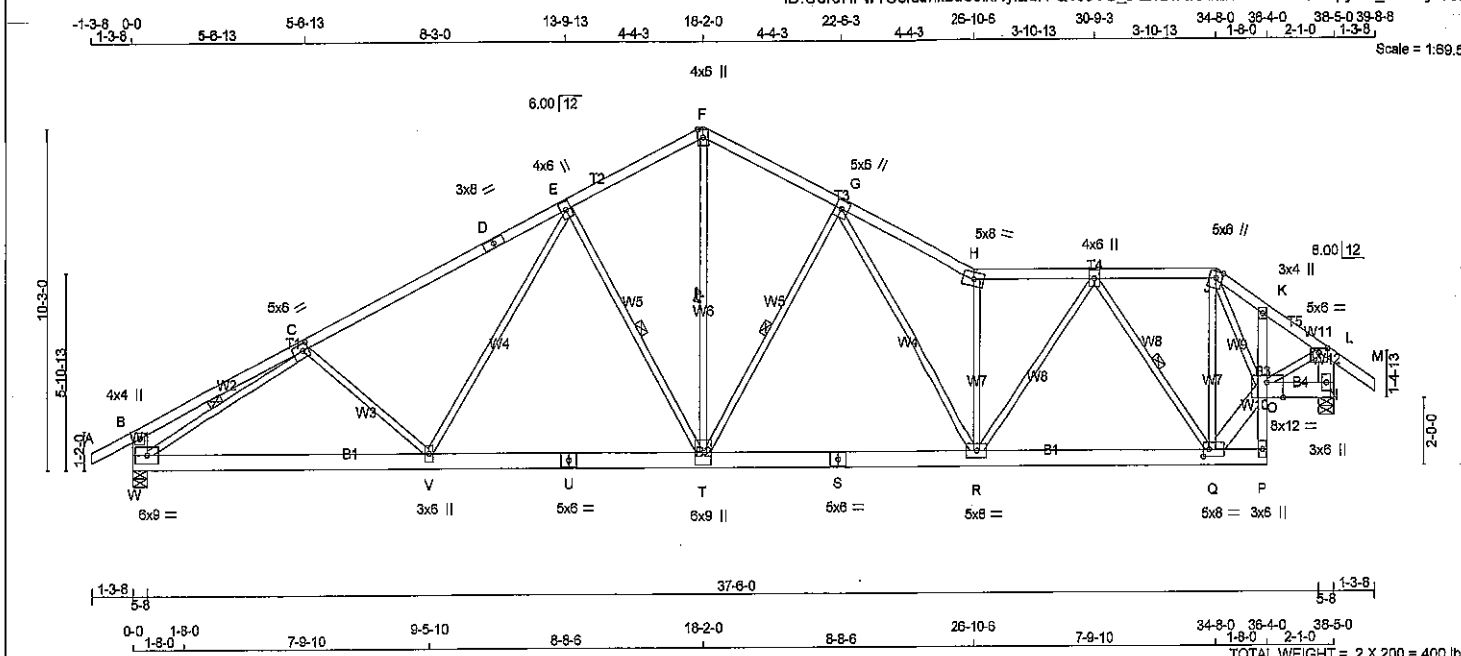
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.88 (C) (INPUT = 1.00)



DWG NO. TAM 45634-17
STRUCTURAL
COMPONENT ONLY



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - J	2x4	DRY	No.2
J - M	2x4	DRY	No.2
W - B	2x6	DRY	No.2
N - L	2x6	DRY	No.2
W - U	2x6	DRY	No.2
U - S	2x6	DRY	No.2
S - P	2x6	DRY	No.2
P - K	2x4	DRY	No.2
O - N	2x6	DRY	No.2

ALL WEBS	2x3	DRY	No.2
EXCEPT			
Q - O	2x6	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	4.0	4.0	
C	TMWW-t	MT20	5.0	6.0	2.50 2.25
D	TS-t	MT20	3.0	8.0	
E	TMWW+t	MT20	4.0	6.0	
F	TTW+p	MT20	4.0	6.0	Edge
G	TMWW+t	MT20	5.0	6.0	
H	TTW-m	MT20	5.0	8.0	
I	TMWW+t	MT20	4.0	6.0	
J	TTWW+m	MT20	5.0	6.0	Edge
K	TMV+p	MT20	3.0	4.0	
L	TMVW-p	MT20	5.0	6.0	1.50 3.00
N	BMV1+p	MT20	3.0	6.0	
O	BMVWVW-t	MT20	8.0	12.0	Edge 6.00
P	BMV+p	MT20	3.0	6.0	
Q	BMVWVW-t	MT20	5.0	8.0	2.50 2.25
R	BMVWVW-t	MT20	5.0	8.0	
S	BS-t	MT20	5.0	6.0	
T	BMVWVW+t	MT20	6.0	9.0	
U	BS-t	MT20	5.0	8.0	
V	BMVWVW-t	MT20	3.0	6.0	
W	BMVWVW-t	MT20	6.0	9.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER							
BEARINGS							
	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG			
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
N	2272	0	2272	0	0	5-8	5-8
W	2271	0	2271	0	0	5-8	5-8

UNFACTORED REACTIONS							
	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1849	1053 / 0	403 / 0	0 / 0	0 / 0	392 / 0	0 / 0
W	1848	1052 / 0	403 / 0	0 / 0	0 / 0	392 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, W

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.28 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-T, G-T, I-Q, C-W. *P-T*

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (L)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (L)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 26	-84.3 -84.3	0.11 (1)	10.00	C-V	-238 / 133	0.11 (1)
B-C	0 / 34	-84.3 -84.3	0.57 (1)	10.00	V-E	0 / 598	0.12 (2)
C-D	-3060 / 0	-84.3 -84.3	0.98 (1)	3.23	E-T	-828 / 0	0.44 (1)
D-E	-3060 / 0	-84.3 -84.3	0.98 (1)	3.23	T-F	0 / 1882	0.42 (1)
E-F	-2405 / 0	-84.3 -84.3	0.50 (1)	3.91	F-G	-1028 / 0	0.54 (1)
F-G	-2384 / 0	-84.3 -84.3	0.28 (1)	4.22	G-R	0 / 1117	0.25 (1)
G-H	-3535 / 0	-84.3 -84.3	0.37 (1)	3.47	R-H	-1854 / 0	0.93 (1)
H-I	-3147 / 0	-84.3 -84.3	0.28 (1)	3.73	R-I	0 / 1312	0.30 (1)
I-J	-1387 / 0	-84.3 -84.3	0.19 (1)	5.33	I-Q	-1796 / 0	0.55 (1)
J-K	-2155 / 0	-84.3 -84.3	0.10 (1)	4.58	Q-J	0 / 152	0.03 (3)
K-L	-2190 / 0	-84.3 -84.3	0.12 (1)	4.52	Q-O	0 / 2180	0.25 (1)
L-M	0 / 32	-84.3 -84.3	0.11 (1)	10.00	J-O	0 / 1022	0.23 (1)
W-B	-269 / 0	0.0 0.0	0.02 (1)	7.81	O-L	0 / 1990	0.45 (1)
N-L	-2241 / 0	0.0 0.0	0.15 (1)	6.84	W-C	-3463 / 0	0.97 (1)
W-V	0 / 2940	-28.0 -28.0	0.51 (1)	10.00			
V-U	0 / 2524	-28.0 -28.0	0.48 (2)	10.00			
U-T	0 / 2524	-28.0 -28.0	0.46 (2)	10.00			
T-S	0 / 2623	-28.0 -28.0	0.45 (1)	10.00			
S-R	0 / 2623	-28.0 -28.0	0.45 (1)	10.00			
R-Q	0 / 2404	-28.0 -28.0	0.41 (1)	10.00			
Q-P	0 / 24	-28.0 -28.0	0.15 (2)	10.00			
P-O	-100 / 0	0.0 0.0	0.05 (1)	7.81			
O-K	-133 / 11	0.0 0.0	0.05 (1)	7.81			
O-N	0 / 0	-28.0 -28.0	0.02 (2)	10.00			

DESIGN CRITERIA	
SPECIFIED LOADS:	
TOP CH. LL	= 25.8 PSF
DL	= 3.0 PSF
BOT CH. LL	= 10.5 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 46.1 PSF

SPACING = 24.0 IN./CIC

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= 1/360 (1.28")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.23")
ALLOWABLE DEFL.(TL)= 1/360 (1.28")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.39")

CSI: TC=0.98 (C-E:1), BC=0.51 (V-W:1), WB=0.97 (C-W:1), SSI=0.25 (C-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

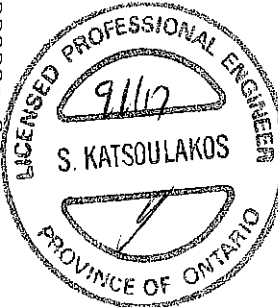
NAIL VALUES					
PLATE	GRIP(DRY)	SHEAR	SECTION		
	(PSI)	(PLI)	(PLI)		
	MAX	MIN	MAX	MIN	MAX
MT20	618	354	1867	822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

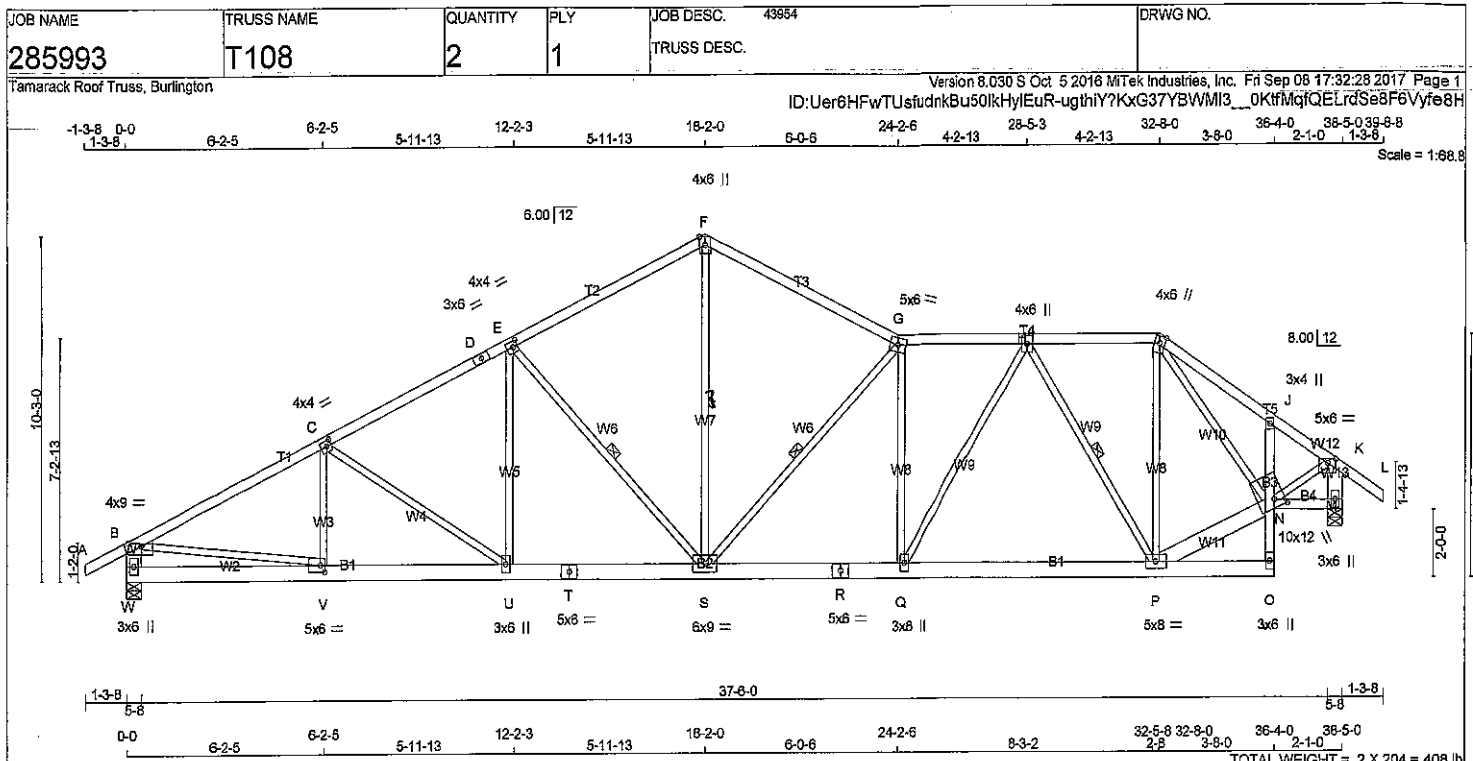
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.85 (C) (INPUT = 1.00)

DWG NO. TAM 45635-17
STRUCTURAL
COMPONENT ONLY



DWG NO. TAM 45635-17
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 2 X 204 = [M/F]

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - L	2x4	DRY	No.2
W - B	2x6	DRY	No.2
M - K	2x8	DRY	No.2
W - T	2x6	DRY	No.2
T - R	2x6	DRY	No.2
R - O	2x6	DRY	No.2
O - J	2x4	DRY	No.2
N - M	2x4	DRY	No.2
ALL WEBS EXCEPT P - N	2x3	DRY	No.2
	2x6	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-p	MT20	4.0	9.0	1.00 4.75
C	TMVW-t	MT20	4.0	4.0	2.00 1.75
D	TS-t	MT20	3.0	6.0	
E	TMVW-t	MT20	4.0	4.0	2.00 1.75
F	TTW-p	MT20	4.0	6.0	Edge
G	TTW-m	MT20	5.0	8.0	
H	TMVW-t	MT20	4.0	8.0	
I	TTW-m	MT20	4.0	8.0	Edge
J	TMV-p	MT20	3.0	4.0	
K	TMVW-p	MT20	5.0	8.0	1.50 3.00
M	BMV-t+p	MT20	3.0	6.0	
N	BVMW-w	MT20	10.0	12.0	3.75 3.00
O	BMV-p	MT20	3.0	6.0	
P	BMVW-t	MT20	5.0	8.0	
Q	BMVW-t	MT20	3.0	6.0	
R	BS-t	MT20	5.0	6.0	
S	BMVW-t	MT20	6.0	9.0	
T	BS-t	MT20	5.0	6.0	
U	BMVW-t	MT20	3.0	6.0	
V	BMVW-t	MT20	5.0	8.0	2.50 1.75
W	BMV-t+p	MT20	3.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
		FACTORED		MAXIMUM FACTORED		INPUT		REORD	
		GROSS REACTION		GROSS REACTION		BRG		BRG	
JT		VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
W	2271	0		2271	0	0	5-8	5-8	
M	2272	0		2272	0	0	5-8	5-8	
UNFACTORED REACTIONS									
		1ST LCASE		MAX/MIN COMPONENT REACTIONS					
JT		COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	
W	1848	1052 / 0		403 / 0	0 / 0	0 / 0	392 / 0	0 / 0	
M	1849	1053 / 0		403 / 0	0 / 0	0 / 0	392 / 0	0 / 0	
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M									
BRACING									
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.37 FT.									
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.									
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.									
1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-S, G-S, H-P. <i>PSP</i>									
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW									
LOADING									
TOTAL LOAD CASES: (4)									

CHORDS				WEBS			
MEMB.	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)
FR-TO		FROM TO		FR-TO		FROM TO	
A-B	0 / 28	-84.3 -84.3	0.11 (1)	V-C	-213 / 130	0.05 (1)	
B-C	-3295 / 0	-84.3 -84.3	0.64 (1)	C-U	-349 / 0	0.32 (1)	
C-D	-2990 / 0	-84.3 -84.3	0.50 (1)	U-E	0 / 436	0.10 (2)	
D-E	-2990 / 0	-84.3 -84.3	0.50 (1)	E-S	-853 / 0	0.49 (1)	
E-F	-2373 / 0	-84.3 -84.3	0.49 (1)	S-F	0 / 1635	0.37 (1)	
F-G	-2350 / 0	-84.3 -84.3	0.51 (1)	S-G	-1097 / 0	0.63 (1)	
G-H	-2819 / 0	-84.3 -84.3	0.29 (1)	Q-G	-621 / 0	0.53 (1)	
H-I	-1584 / 0	-84.3 -84.3	0.22 (1)	P-I	0 / 639	0.14 (1)	
I-J	-2142 / 0	-84.3 -84.3	0.19 (1)	P-N	0 / 1760	0.21 (1)	
J-K	-2139 / 0	-84.3 -84.3	0.12 (1)	I-N	0 / 337	0.08 (1)	
K-L	0 / 32	-84.3 -84.3	0.11 (1)	B-V	0 / 2989	0.67 (1)	
W-B	-2183 / 0	0.0 0.0	0.14 (1)	N-K	0 / 1993	0.45 (1)	
M-K	-2241 / 0	0.0 0.0	0.15 (1)	Q-H	0 / 945	0.21 (1)	
				H-P	-1448 / 0	0.62 (1)	
W-V	0 / 0	-28.0 -28.0	0.12 (2)				
V-U	0 / 2986	-28.0 -28.0	0.41 (1)				
U-T	0 / 2675	-28.0 -28.0	0.37 (1)				
T-S	0 / 2875	-28.0 -28.0	0.37 (1)				
S-R	0 / 2826	-28.0 -28.0	0.39 (1)				
R-Q	0 / 2828	-28.0 -28.0	0.39 (1)				
Q-P	0 / 2331	-28.0 -28.0	0.38 (1)				
P-O	0 / 19	-28.0 -28.0	0.17 (2)				
O-N	0 / 22	0.0 0.0	0.05 (1)				
N-J	-283 / 0	0.0 0.0	0.04 (1)				
N-M	0 / 0	-28.0 -28.0	0.04 (2)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.28")
CALCULATED VERT. DEFL.(LL) = L/999 (0.19")
ALLOWABLE DEFL.(TL) = L/360 (1.28")
CALCULATED VERT. DEFL.(TL) = L/999 (0.31")

CSI: TC=0.64 (B-C-1), BC=0.41 (U-V-1), WB=0.67 (B-V-1), SSI=0.21 (B-C-1)

DOI LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667 622 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.74 (V) (INPUT = 1.00)

DWG NO. TAM45636-17
STRUCTURAL COMPONENT ONLY



DRWG NO. TAW45636 17
STRUCTURAL COMPONENT ONLY

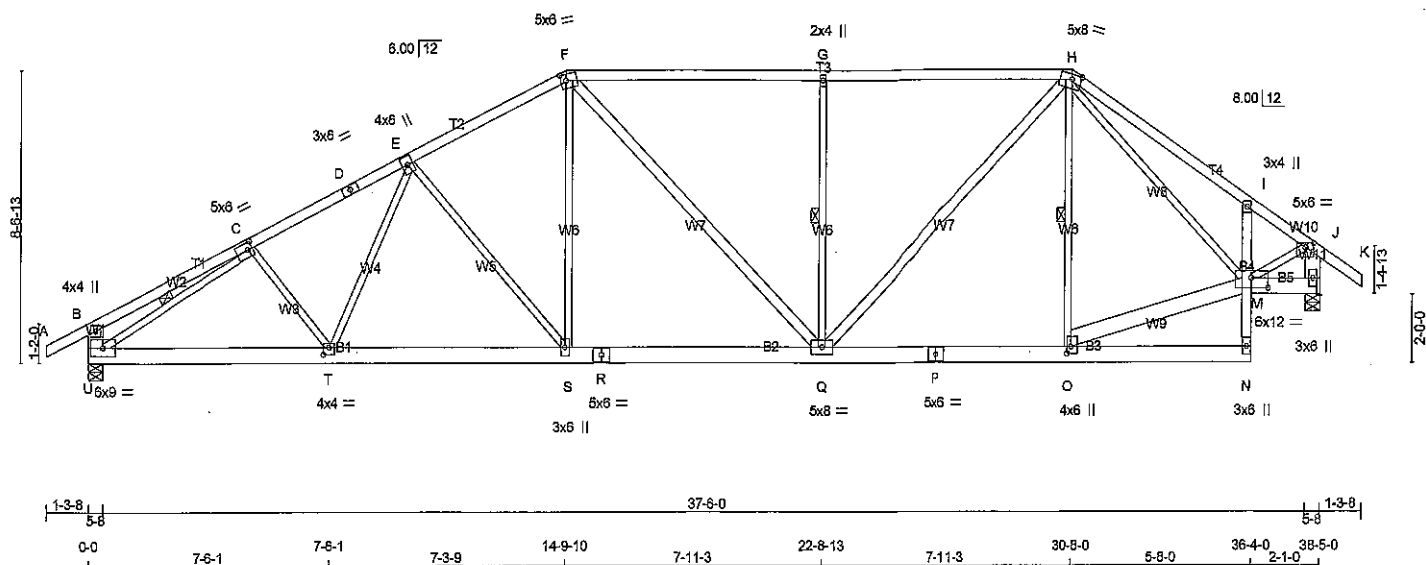
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
285993	T109	6	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:Uer6HFwTUusfndkBu50kHylEuR-ugthY?KxG37YBWMI3_0K10MqesEJQdSe8F6Vye8H

-1-3-8 0-0 5-0-14 4-10-6 0-11-4 4-10-8 14-9-10 7-11-3 22-8-13 7-11-3 30-8-0 5-8-0 35-4-0 38-5-0 39-8-8 1-3-8
Scale = 1:87.6



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - H	2x4	DRY	No.2	SPF	
H - K	2x4	DRY	No.2	SPF	
U - B	2x6	DRY	No.2	SPF	
L - J	2x6	DRY	No.2	SPF	
U - R	2x6	DRY	No.2	SPF	
R - P	2x6	DRY	No.2	SPF	
P - N	2x6	DRY	No.2	SPF	
N - I	2x4	DRY	No.2	SPF	
M - L	2x6	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	
F - Q	2x4	DRY	No.2	SPF	
Q - H	2x4	DRY	No.2	SPF	
O - M	2x6	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	1.75
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	6.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMW+w	MT20	2.0	4.0		
H	TTWWV-m	MT20	5.0	8.0	1.75	3.50
J	TMV+p	MT20	3.0	4.0		
L	TMV+p	MT20	5.0	6.0	1.50	3.00
M	BMV1+p	MT20	3.0	6.0		
N	BMV+p	MT20	6.0	12.0	3.50	6.00
O	BMV+p	MT20	3.0	6.0		
P	BMWW-t	MT20	4.0	6.0	2.50	1.50
Q	BS-t	MT20	5.0	6.0		
R	BMWW-t	MT20	5.0	8.0		
Q	BS-t	MT20	5.0	6.0		
S	BMWW-t	MT20	3.0	6.0		
T	BMWW-t	MT20	4.0	4.0	2.50	2.00
U	BMV1-t	MT20	6.0	9.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
L	2272	0	2272	0	5-8	5-8
U	2271	0	2271	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
L	1849	1053 / 0	403 / 0	0 / 0	0 / 0	392 / 0	0 / 0
U	1848	1052 / 0	403 / 0	0 / 0	0 / 0	392 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, U

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-Q, H-O, C-U.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM	TO			FR-TO			
A-B	0 / 26	-84.3	-84.3	0.11 (1)	10.00	C-T	-1 / 166	0.04 (3)	
B-C	0 / 18	-84.3	-84.3	0.25 (1)	10.00	T-E	0 / 310	0.07 (2)	
C-D	-3211 / 0	-84.3	-84.3	0.37 (1)	3.64	E-S	-579 / 0	0.66 (1)	
D-E	-3211 / 0	-84.3	-84.3	0.37 (1)	3.64	S-F	0 / 665	0.15 (1)	
E-F	-2711 / 0	-84.3	-84.3	0.34 (1)	3.95	F-Q	0 / 171	0.03 (1)	
F-G	-2534 / 0	-84.3	-84.3	0.89 (1)	3.20	Q-G	-825 / 0	0.37 (1)	
G-H	-2534 / 0	-84.3	-84.3	0.89 (1)	3.20	Q-H	0 / 1267	0.20 (1)	
H-I	-2276 / 0	-84.3	-84.3	0.43 (1)	4.15	O-H	-354 / 70	0.16 (1)	
I-J	-2248 / 0	-84.3	-84.3	0.28 (1)	4.30	O-M	0 / 1722	0.20 (1)	
J-K	0 / 32	-84.3	-84.3	0.11 (1)	10.00	H-M	0 / 327	0.07 (1)	
U-B	-289 / 0	0.0	0.0	0.02 (1)	7.61	U-C	-3438 / 0	0.83 (1)	
L-J	-2241 / 0	0.0	0.0	0.15 (1)	8.84	M-J	0 / 2099	0.47 (1)	
U-T	0 / 2875	-28.0	-28.0	0.45 (1)	10.00				
T-S	0 / 2786	-28.0	-28.0	0.42 (1)	10.00				
S-R	0 / 2415	-28.0	-28.0	0.38 (1)	10.00				
R-Q	0 / 2415	-28.0	-28.0	0.38 (1)	10.00				
Q-P	0 / 1852	-28.0	-28.0	0.30 (2)	10.00				
P-O	0 / 1852	-28.0	-28.0	0.30 (2)	10.00				
O-N	0 / 34	-28.0	-28.0	0.16 (2)	10.00				
N-M	0 / 97	0.0	0.0	0.10 (1)	10.00				
M-I	-442 / 0	0.0	0.0	0.08 (1)	7.81				
M-L	0 / 0	-28.0	-28.0	0.02 (2)	10.00				

REGISTERED PROFESSIONAL

9/1/12

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 058-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.28")
CALCULATED VERT. DEFL.(LL) = L/999 (0.16")
ALLOWABLE DEFL.(TL) = L/360 (1.28")
CALCULATED VERT. DEFL.(TL) = L/999 (0.27")

CS1 TC=0.89 (F-G-1), BC=0.45 (T-U-1), WB=0.83 (C-U-1), SS1=0.32 (F-G-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSD) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 622 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.89 (F) (INPUT = 0.80)

JSI METAL= 0.85 (C) (INPUT = 1.00)

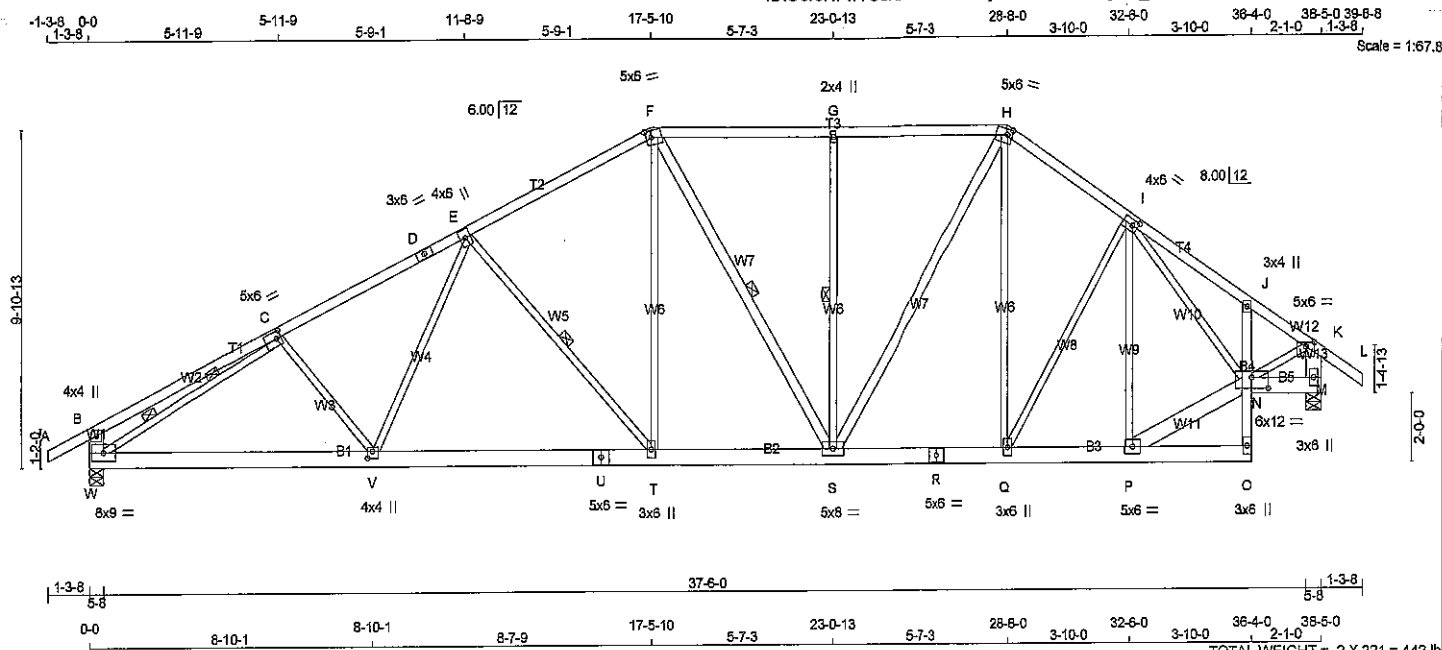


DWG NO. TAM 45637-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285993	T110	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - L	2x4	DRY	No.2
W - B	2x6	DRY	No.2
M - K	2x6	DRY	No.2
W - U	2x6	DRY	No.2
U - R	2x6	DRY	No.2
R - O	2x6	DRY	No.2
O - J	2x4	DRY	No.2
N - M	2x6	DRY	No.2
ALL WEBS 2x3 DRY No.2			
EXCEPT			
F - S	2x4	DRY	No.2
S - H	2x4	DRY	No.2
P - N	2x6	DRY	No.2

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
M	2272	0	2272	0	5-8	5-8
W	2271	0	2271	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
M	1849	1053 / 0	403 / 0	0 / 0	0 / 0	392 / 0	0 / 0
W	1848	1052 / 0	403 / 0	0 / 0	0 / 0	392 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, W

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

- 1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-T, F-S, G-S.
- 2 LATERAL BRACE(S) AT 1/3 LENGTH OF C-W.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		MAX. UNBRACED LENGTH	MEMB.	WEBS	
	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	FROM	TO			MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO								
A-B	0 / 28	-84.3	-84.3	0.11 (1)	10.00	C-V	-138 / 131	0.05 (1)
B-C	0 / 20	-84.3	-84.3	0.38 (1)	10.00	V-E	0 / 479	0.11 (2)
C-D	-3170 / 0	-84.3	-84.3	0.50 (1)	3.58	E-T	-774 / 0	0.41 (1)
D-E	-3170 / 0	-84.3	-84.3	0.50 (1)	3.58	T-F	0 / 788	0.18 (1)
E-F	-2441 / 0	-84.3	-84.3	0.44 (1)	4.02	F-S	-103 / 0	0.07 (2)
F-G	-2124 / 0	-84.3	-84.3	0.40 (1)	4.27	S-G	-576 / 0	0.37 (1)
G-H	-2124 / 0	-84.3	-84.3	0.40 (1)	4.27	H-S	0 / 826	0.15 (1)
H-I	-2013 / 0	-84.3	-84.3	0.20 (1)	4.80	Q-H	0 / 154	0.03 (3)
I-J	-2156 / 0	-84.3	-84.3	0.21 (1)	4.47	Q-I	0 / 138	0.03 (1)
J-K	-2202 / 0	-84.3	-84.3	0.09 (1)	4.54	P-I	-710 / 0	0.64 (1)
K-L	0 / 32	-84.3	-84.3	0.11 (1)	10.00	P-N	0 / 1773	0.21 (1)
W-B	-317 / 0	0.0	0.0	0.02 (1)	7.81	I-N	0 / 372	0.08 (1)
M-K	-2241 / 0	0.0	0.0	0.15 (1)	6.84	W-C	-3457 / 0	1.00 (1)
						N-K	0 / 2014	0.45 (1)
W-V	0 / 2922	-28.0	-28.0	0.49 (2)	10.00			
V-U	0 / 2675	-28.0	-28.0	0.47 (2)	10.00			
U-T	0 / 2675	-28.0	-28.0	0.47 (2)	10.00			
T-S	0 / 2172	-28.0	-28.0	0.34 (1)	10.00			
S-R	0 / 1654	-28.0	-28.0	0.24 (1)	10.00			
R-Q	0 / 1654	-28.0	-28.0	0.24 (1)	10.00			
Q-P	0 / 1586	-28.0	-28.0	0.24 (1)	10.00			
P-O	0 / 28	-28.0	-28.0	0.06 (2)	10.00			
O-N	0 / 70	0.0	0.0	0.06 (1)	10.00			
N-J	-212 / 0	0.0	0.0	0.08 (1)	7.81			
N-M	0 / 0	-28.0	-28.0	0.02 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 25.6	PSF
DL	= 3.0	PSF	
BOT CH.	LL	= 10.5	PSF
DL	= 7.0	PSF	
TOTAL LOAD	= 46.1	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.28")
CALCULATED VERT. DEFL.(LL) = L/999 (0.17")
ALLOWABLE DEFL.(TL) = L/360 (1.28")
CALCULATED VERT. DEFL.(TL) = L/999 (0.28")

CSI: TC=0.50 (C-E:1), BC=0.49 (V-W:2), WB=1.00 (C-W:1), SSI=0.23 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (H) (INPUT = 0.90)
JSI METAL= 0.85 (C) (INPUT = 1.00)



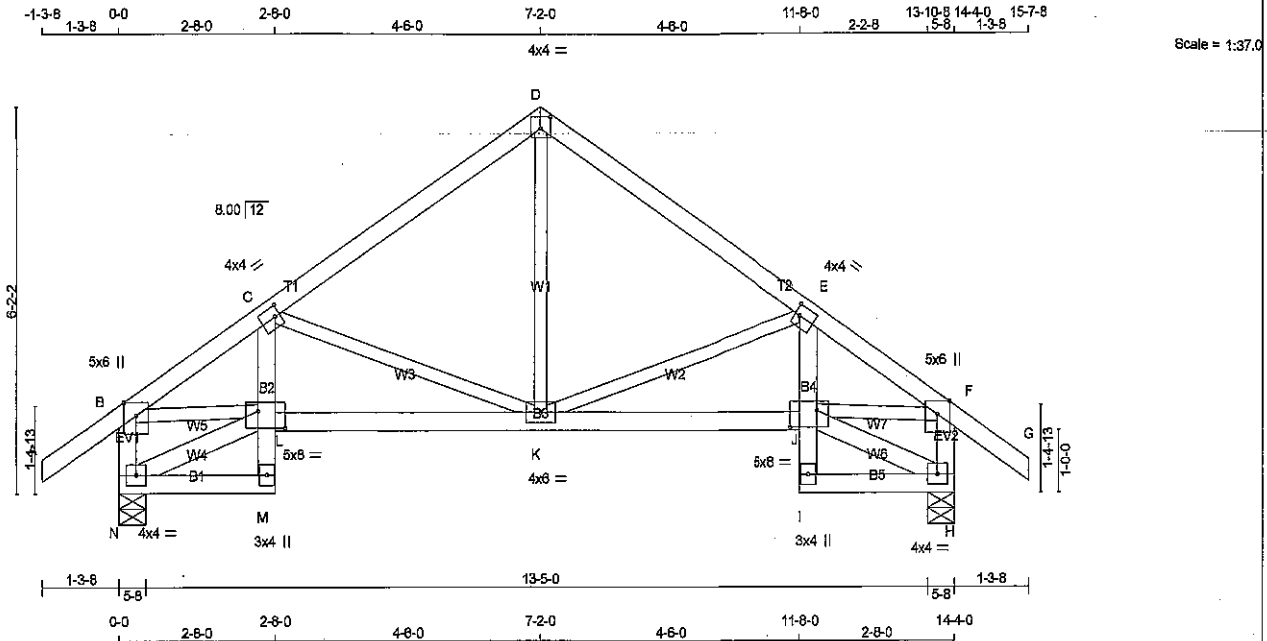
DWG NO. TAM 45638.17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
285994	T112	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1:37.0

TOTAL WEIGHT = 2 X 68 = 136 lb

LUMBER					
N. L. G. A. RULES					
CHORDS		SIZE	LUMBER	DESCR.	
A - D	2x4	DRY	No.2	SPF	
D - G	2x4	DRY	No.2	SPF	
N - B	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
N - M	2x4	DRY	No.2	SPF	
M - C	2x4	DRY	No.2	SPF	
L - J	2x4	DRY	No.2	SPF	
I - E	2x4	DRY	No.2	SPF	
I - H	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT		2x3	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMVW-t	MT20	4.0	4.0	2.00	1.00
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMVW-t	MT20	4.0	4.0	2.00	1.00
F	TMVW+p	MT20	5.0	6.0	Edge	
H	BMVW-t	MT20	4.0	4.0		
I	BMV+p	MT20	3.0	4.0		
J	BMVW-t	MT20	5.0	8.0	3.25	5.50
K	BMVW-t	MT20	4.0	6.0		
L	BMVW-t	MT20	5.0	8.0	3.25	5.50
M	BMV+p	MT20	3.0	4.0		
N	BMVW-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
N	920	0	920	0	0	5-8	5-8	5-8	5-8
H	920	0	920	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	739	437 / 0	150 / 0	0 / 0	0 / 0	152 / 0	0 / 0
H	739	437 / 0	150 / 0	0 / 0	0 / 0	152 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.60 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CS (LC)	MEMB.	FORCE (LBS)	MAX. CS (LC)	MAX. CS (LC)
FR-TO		FROM TO		FR-TO			
A-B	0 / 32	-84.3 -84.3	0.11 (1)	10.00	K-D	0 / 482	0.11 (1)
B-C	-1234 / 0	-84.3 -84.3	0.17 (1)	5.80	K-E	-500 / 0	0.20 (1)
C-D	-748 / 0	-84.3 -84.3	0.22 (1)	6.25	C-K	-500 / 0	0.20 (1)
D-E	-748 / 0	-84.3 -84.3	0.22 (1)	6.25	N-L	-40 / 0	0.00 (1)
E-F	-1234 / 0	-84.3 -84.3	0.17 (1)	5.60	S-L	0 / 1053	0.24 (1)
F-G	0 / 32	-84.3 -84.3	0.11 (1)	10.00	J-H	-40 / 0	0.00 (1)
N-B	-869 / 0	0.0 0.0	0.09 (1)	7.81	J-F	0 / 1053	0.24 (1)
H-F	-869 / 0	0.0 0.0	0.09 (1)	7.81			
N-M	0 / 37	-28.0 -28.0	0.06 (2)	10.00			
M-L	0 / 62	0.0 0.0	0.08 (1)	10.00			
L-C	0 / 147	0.0 0.0	0.07 (1)	10.00			
L-K	0 / 1076	-28.0 -28.0	0.30 (2)	10.00			
K-J	0 / 1076	-28.0 -28.0	0.30 (2)	10.00			
I-J	0 / 62	0.0 0.0	0.06 (1)	10.00			
J-E	0 / 147	0.0 0.0	0.07 (1)	10.00			
I-H	0 / 37	-28.0 -28.0	0.06 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.48")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.48")
CALCULATED VERT. DEFL.(TL) = L/999 (0.06")

CSI: TC=0.22/1.00 (D-E:1), BC=0.30/1.00 (J-K:2), WB=0.24/1.00 (F-J:1), SS=0.15/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (L) (INPUT = 0.90)
JSI METAL= 0.24 (F) (INPUT = 1.00)



DRWG NO. TAM 2/226-18
STRUCTURAL
COMPONENT ONLY

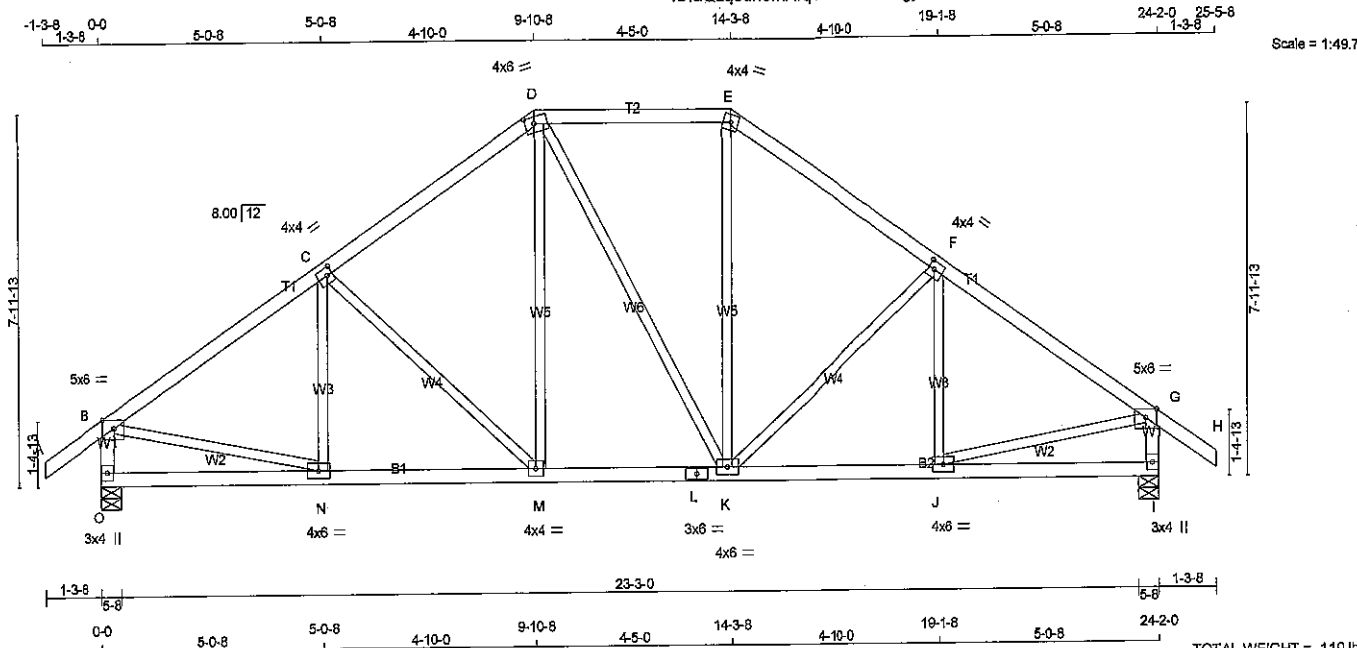
PROVINCE OF ONTARIO
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
285993	T132	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
E - H	2x4	DRY	No.2	SPF	
O - B	2x4	DRY	No.2	SPF	
I - G	2x4	DRY	No.2	SPF	
O - L	2x4	DRY	No.2	SPF	
L - I	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMWV-p	MT20	5.0	6.0	Edge	
C	TMWV-t	MT20	4.0	4.0	2.00 1.50	
D	TTWV-m	MT20	4.0	6.0	1.75 2.50	
E	TTWV-m	MT20	4.0	4.0		
F	TMWV-t	MT20	4.0	4.0	2.00 1.50	
G	TMWV-t	MT20	5.0	6.0	Edge	
I	BMV1+p	MT20	3.0	4.0		
J	BMWV-t	MT20	4.0	6.0		
K	BMWV-t	MT20	4.0	6.0		
L	BS-t	MT20	3.0	6.0		
M	BMWV-t	MT20	4.0	4.0		
N	BMWV-t	MT20	4.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	HORZ	UPLIFT
O	1472	0	1472	0
I	1472	0	1472	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN	COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1192	689 / 0	254 / 0	0 / 0	0 / 0	250 / 0	0 / 0	250 / 0	0 / 0
I	1192	689 / 0	254 / 0	0 / 0	0 / 0	250 / 0	0 / 0	250 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.03 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MAX. UNBRACED LENGTH (FT)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO						FR-TO			
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	N-C	-120 / 142	0.04 (1)	
B-C	-1520 / 0	-84.3	-84.3	0.30 (1)	5.03	C-M	-342 / 0	0.28 (1)	
C-D	-1271 / 0	-84.3	-84.3	0.28 (1)	5.40	M-D	0 / 374	0.08 (2)	
D-E	-1039 / 0	-84.3	-84.3	0.22 (1)	5.90	D-K	0 / 2	0.00 (2)	
E-F	-1272 / 0	-84.3	-84.3	0.28 (1)	5.40	K-E	0 / 376	0.08 (2)	
F-G	-1520 / 0	-84.3	-84.3	0.30 (1)	5.03	K-F	-340 / 0	0.27 (1)	
G-H	0 / 32	-84.3	-84.3	0.11 (1)	10.00	J-F	-121 / 140	0.04 (1)	
O-B	-1414 / 0	0.0	0.0	0.15 (1)	6.85	B-N	0 / 1316	0.30 (1)	
I-G	-1413 / 0	0.0	0.0	0.15 (1)	6.88	J-G	0 / 1316	0.30 (1)	
O-N	0 / 0	-28.0	-28.0	0.17 (3)	10.00				
N-M	0 / 1287	-28.0	-28.0	0.32 (2)	10.00				
M-L	0 / 1038	-28.0	-28.0	0.23 (2)	10.00				
L-K	0 / 1038	-28.0	-28.0	0.23 (2)	10.00				
K-J	0 / 1287	-28.0	-28.0	0.32 (2)	10.00				
J-I	0 / 0	-28.0	-28.0	0.17 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/999 (0.09")

CS1: TC=0.30 (B-C:1), BC=0.32 (M-N:2), WB=0.30 (B-N:1), SS=0.17 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 622 2284 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (N) (INPUT = 0.90)
JSI METAL = 0.32 (N) (INPUT = 1.00)



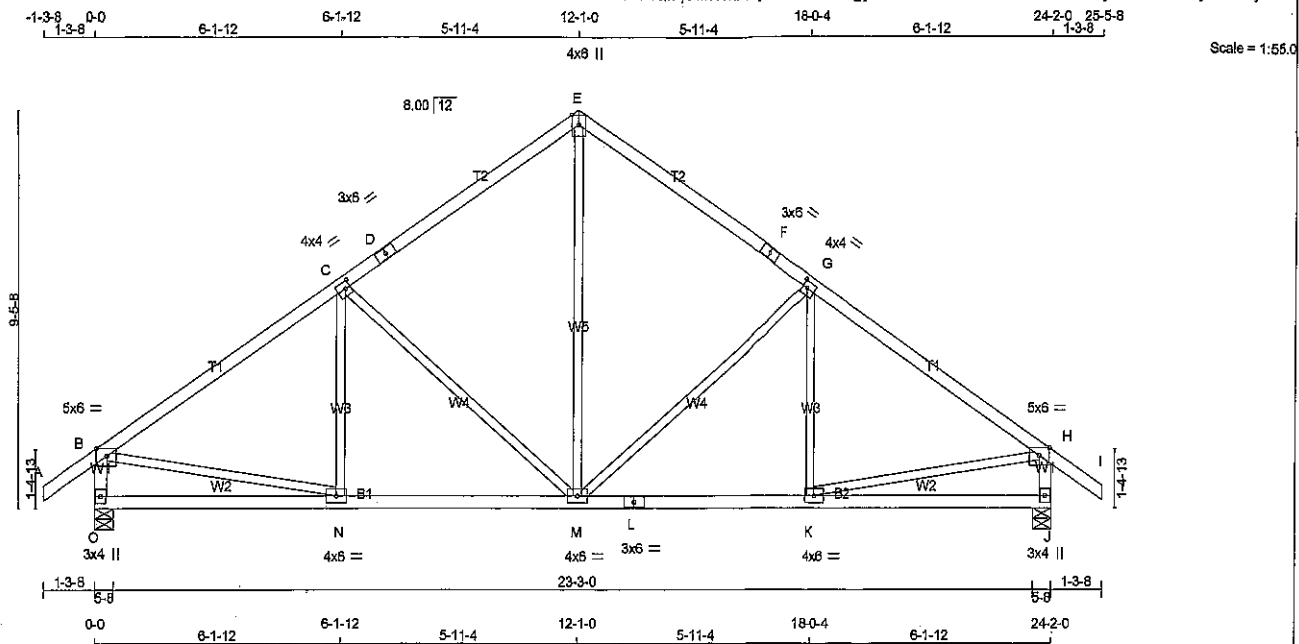
DWG NO. TAM 4564217
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
285993	T133	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
E - F	2x4	DRY	No.2	SPF	
F - I	2x4	DRY	No.2	SPF	
O - B	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
O - L	2x4	DRY	No.2	SPF	
L - J	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVV-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTW+p	MT20	4.0	6.0	Edge	
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TMVV-p	MT20	5.0	6.0	Edge	
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	4.0	6.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	6.0		
N	BMWW-t	MT20	4.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	IN-SX	IN-SX
O	1472	0	5-8	5-8
J	1472	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS
JT	COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL
O	1192 689 / 0 254 / 0 0 / 0 0 / 0 250 / 0 0 / 0
J	1192 689 / 0 254 / 0 0 / 0 0 / 0 250 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.82 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM	TO		FR-TO			
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	M-E	0 / 831	0.19 (1)
B-C	-1518 / 0	-84.3	-84.3	0.46 (1)	4.82	M-G	-512 / 0	0.66 (1)
C-D	-1122 / 0	-84.3	-84.3	0.42 (1)	5.46	K-G	-38 / 224	0.05 (3)
D-E	-1122 / 0	-84.3	-84.3	0.42 (1)	5.46	C-M	-512 / 0	0.66 (1)
E-F	-1122 / 0	-84.3	-84.3	0.42 (1)	5.46	N-C	-38 / 224	0.05 (3)
F-G	-1122 / 0	-84.3	-84.3	0.42 (1)	5.46	B-N	0 / 1311	0.29 (1)
G-H	-1518 / 0	-84.3	-84.3	0.46 (1)	4.82	K-H	0 / 1311	0.29 (1)
H-I	0 / 32	-84.3	-84.3	0.11 (1)	10.00			
O-B	-1402 / 0	0.0	0.0	0.14 (1)	6.88			
J-H	-1402 / 0	0.0	0.0	0.14 (1)	6.88			
C-N	0 / 0	-28.0	-28.0	0.25 (3)	10.00			
N-M	0 / 1291	-28.0	-28.0	0.40 (2)	10.00			
M-L	0 / 1291	-28.0	-28.0	0.40 (2)	10.00			
L-K	0 / 1291	-28.0	-28.0	0.40 (2)	10.00			
K-J	0 / 0	-28.0	-28.0	0.25 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 3.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CSI: TC=0.46 (B-C:1), BC=0.40 (M-N:2), WB=0.66 (O-M:1), SS=0.21 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	818 354	1857 822	2284 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (N) (INPUT = 0.90)
JSI METAL= 0.35 (L) (INPUT = 1.00)

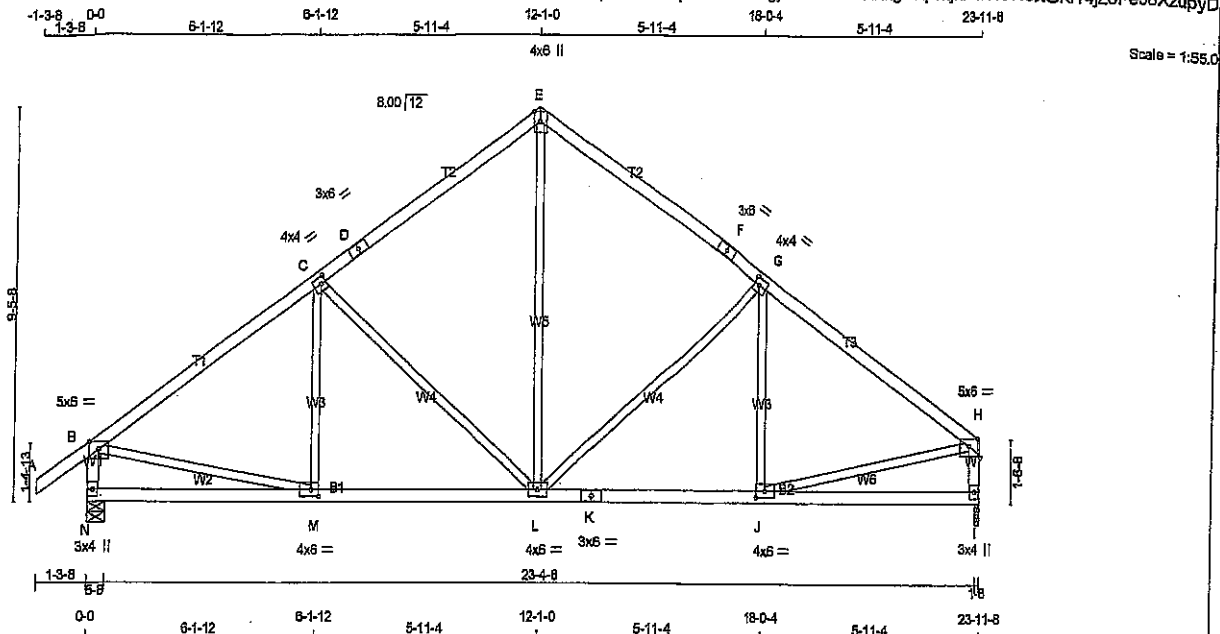


DRWG NO. TAM 45643-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
285930	T133A	6	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 6 X 102 = 609 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
E - F	2x4	DRY	No.2	SPF	
F - H	2x4	DRY	No.2	SPF	
N - B	2x4	DRY	No.2	SPF	
I - H	2x4	DRY	No.2	SPF	
N - K	2x4	DRY	No.2	SPF	
K - I	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table ts in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTW+p	MT20	4.0	8.0	Edge	
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW-p	MT20	5.0	6.0	Edge	
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	4.0	6.0	2.00	2.75
K	BS-t	MT20	3.0	8.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMWW-t	MT20	4.0	6.0	2.00	2.50
N	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
N	1460	0	1460	0	0	5-8	5-8		
I	1345	0	1345	0	0	1-8	1-8		

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN.	COMPONENT REACTIONS	WIND	DEAD	SOIL
N	1163	683 / 0	252 / 0	0 / 0	0 / 0	248 / 0
I	1104	613 / 0	252 / 0	0 / 0	0 / 0	240 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.85 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		W E B S	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX (PLF)	MAX. UNBRAC LENGTH	MEMB. FORCE (LBS)
FR-TO					
A-B	0 / 32	-84.3	-84.3 0.11 (1)	10.00	L-E 0 / 809 0.18 (1)
B-C	-1501 / 0	-84.3	-84.3 0.46 (1)	4.85	L-G -474 / 0 0.61 (1)
C-D	-1104 / 0	-84.3	-84.3 0.42 (1)	5.49	J-G -71 / 202 0.05 (3)
D-E	-1104 / 0	-84.3	-84.3 0.42 (1)	5.49	C-L -514 / 0 0.66 (1)
E-F	-1103 / 0	-84.3	-84.3 0.41 (1)	5.52	M-C -38 / 225 0.05 (3)
F-G	-1103 / 0	-84.3	-84.3 0.41 (1)	5.52	B-M 0 / 1297 0.29 (1)
G-H	-1485 / 0	-84.3	-84.3 0.43 (1)	4.83	J-H 0 / 1273 0.28 (1)
N-B	-1390 / 0	0.0	0.0 0.14 (1)	6.80	
I-H	-1277 / 0	0.0	0.0 0.13 (1)	7.13	
N-M	0 / 0	-28.0	-28.0 0.25 (3)	10.00	
M-L	0 / 1277	-28.0	-28.0 0.40 (2)	10.00	
L-K	0 / 1247	-28.0	-28.0 0.38 (2)	10.00	
K-J	0 / 1247	-28.0	-28.0 0.38 (2)	10.00	
J-I	0 / 0	-28.0	-28.0 0.24 (3)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.8 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 085-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.80")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.07")
ALLOWABLE DEFL.(TL) = $L/360$ (0.80")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.11")

CSI: TC=0.48/1.00 (B-C:1), BC=0.40/1.00 (L-M:2),
WB=0.88/1.00 (C-L:1), SS=0.21/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

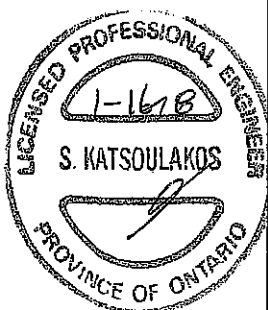
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1867 822 2284 1656

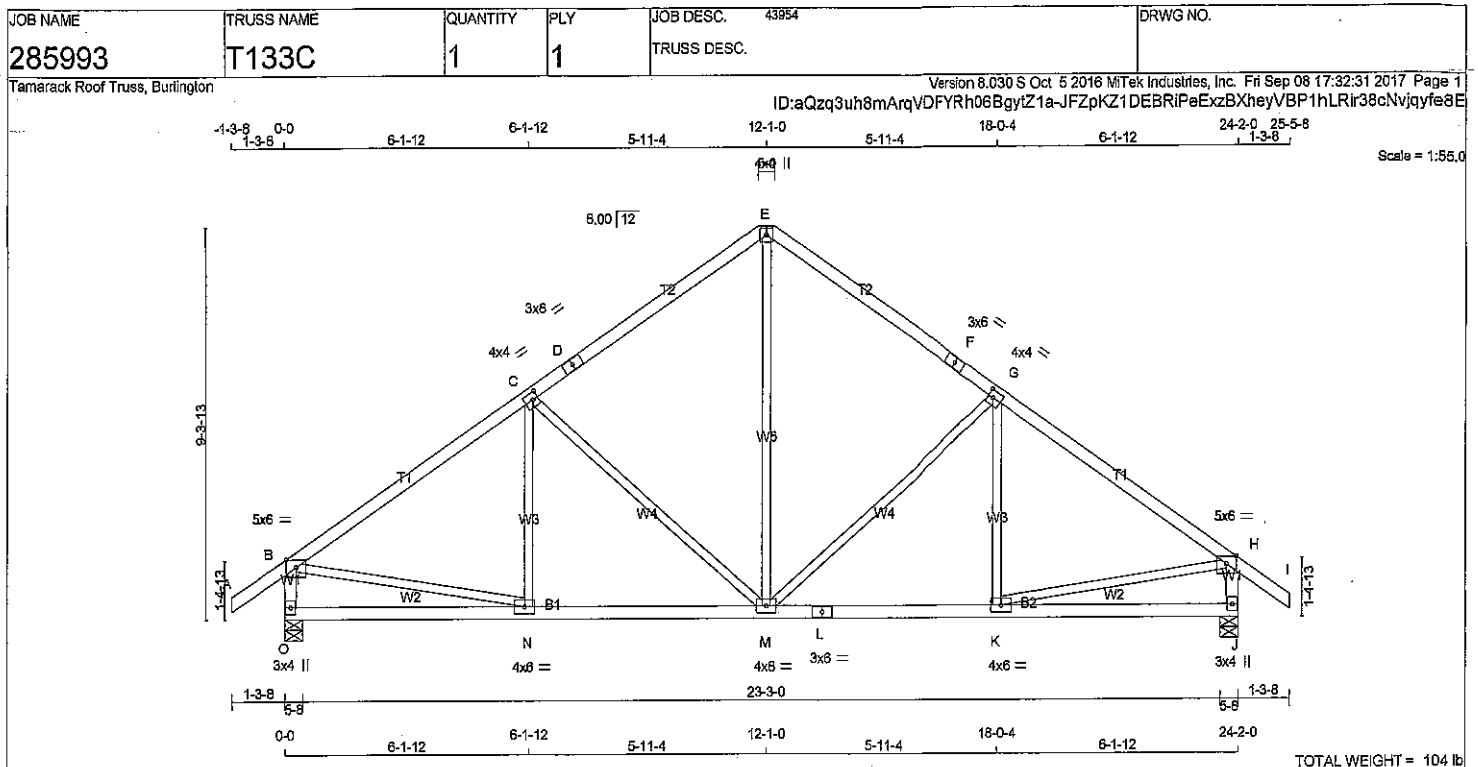
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

.JSI GRIP= 0.84 (J) (INPUT = 0.90)
.JSI METAL= 0.34 (K) (INPUT = 1.00)



DRWG NO. T133A 3982-13
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS SIZE

LUMBER

DESCR.

A - D

2x4

DRY

No.2

SPF

D - E

2x4

DRY

No.2

SPF

E - F

2x4

DRY

No.2

SPF

F - I

2x4

DRY

No.2

SPF

O - B

2x4

DRY

No.2

SPF

J - H

2x4

DRY

No.2

SPF

O - L

2x4

DRY

No.2

SPF

L - J

2x4

DRY

No.2

SPF

ALL WEBS

2x3

DRY

No.2

SPF

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE PLATES W LEN Y X

B TMVW-p MT20 5.0 6.0 Edge

C TMWW-t MT20 4.0 4.0 2.00 1.50

D TS-t MT20 3.0 6.0

E TTW+p MT20 4.0 4.0

F TS-t MT20 3.0 6.0

G TMWW-t MT20 4.0 4.0 2.00 1.50

H TMVW-p MT20 5.0 6.0 Edge

K BMVt+p MT20 3.0 4.0

L BMWW-t MT20 4.0 6.0

M BS-t MT20 3.0 6.0

N BMWWW-t MT20 4.0 6.0

O BMWW-t MT20 4.0 6.0

C BMVt+p MT20 3.0 4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED MAXIMUM FACTORED INPUT REQ'D

GROSS REACTION DOWN HORZ UPLIFT IN-SX IN-SX

JT VERT HORZ

O 1472 0 1472 0 0 5-8 5-8

J 1472 0 1472 0 0 5-8 5-8

UNFACTORED REACTIONS

1ST LCASE MAX/MIN. COMPONENT REACTIONS

JT COMBINED SNOW LIVE PERM. LIVE WIND DEAD SOIL

O 1192 689 / 0 254 / 0 0 / 0 0 / 0 250 / 0 0 / 0

J 1192 689 / 0 254 / 0 0 / 0 0 / 0 250 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.82 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. MAX. FACTORED FORCE (LBS)

FACTORED VERT. LOAD LC1 MAX (PLF) CSI (LC)

FR-TO FROM TO LENGTH FR-TO

A-B 0 / 32 -84.3 -84.3 0.11 (1) 10.00

B-C -1518 / 0 -84.3 -84.3 0.46 (1) 4.82

C-D -1122 / 0 -84.3 -84.3 0.42 (1) 5.46

D-E -1122 / 0 -84.3 -84.3 0.42 (1) 5.46

E-F -1122 / 0 -84.3 -84.3 0.42 (1) 5.46

F-G -1122 / 0 -84.3 -84.3 0.42 (1) 5.46

G-H -1518 / 0 -84.3 -84.3 0.46 (1) 4.82

H-I 0 / 32 -84.3 -84.3 0.11 (1) 10.00

O-B -1402 / 0 0.0 0.0 0.14 (1) 6.88

J-H -1402 / 0 0.0 0.0 0.14 (1) 6.88

O-N 0 / 0 -28.0 -28.0 0.25 (3) 10.00

N-M 0 / 1291 -28.0 -28.0 0.40 (2) 10.00

M-L 0 / 1291 -28.0 -28.0 0.40 (2) 10.00

L-K 0 / 1291 -28.0 -28.0 0.40 (2) 10.00

K-J 0 / 0 -28.0 -28.0 0.25 (3) 10.00

WEBS

MEMB. MAX. FACTORED FORCE (LBS) CSI (LC)

M-E 0 / 831 0.19 (1)

M-G -512 / 0 0.66 (1)

K-G -38 / 224 0.05 (3)

C-M -512 / 0 0.66 (1)

N-C -38 / 224 0.05 (3)

B-N 0 / 1311 0.29 (1)

K-H 0 / 1311 0.29 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 3.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, 8CBC 2012, ABC 2014

- CSA 086-09

- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")

CALCULATED VERT. DEFL.(LL) = L/999 (0.07")

ALLOWABLE DEFL.(TL)= L/360 (0.81")

CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CSI: TC=0.46 (B-C:1), BC=0.40 (M-N:2), WB=0.86 (G-M:1), SSI=0.21 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 618 354 1667 822 2264 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.85 (N) (INPUT = 0.90)

JSI METAL= 0.35 (L) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER

9/1/17

S. KATSOULAKOS

PROVINCE OF ONTARIO

DWG NO. TAM 45645-17

STRUCTURAL

COMPONENT ONLY



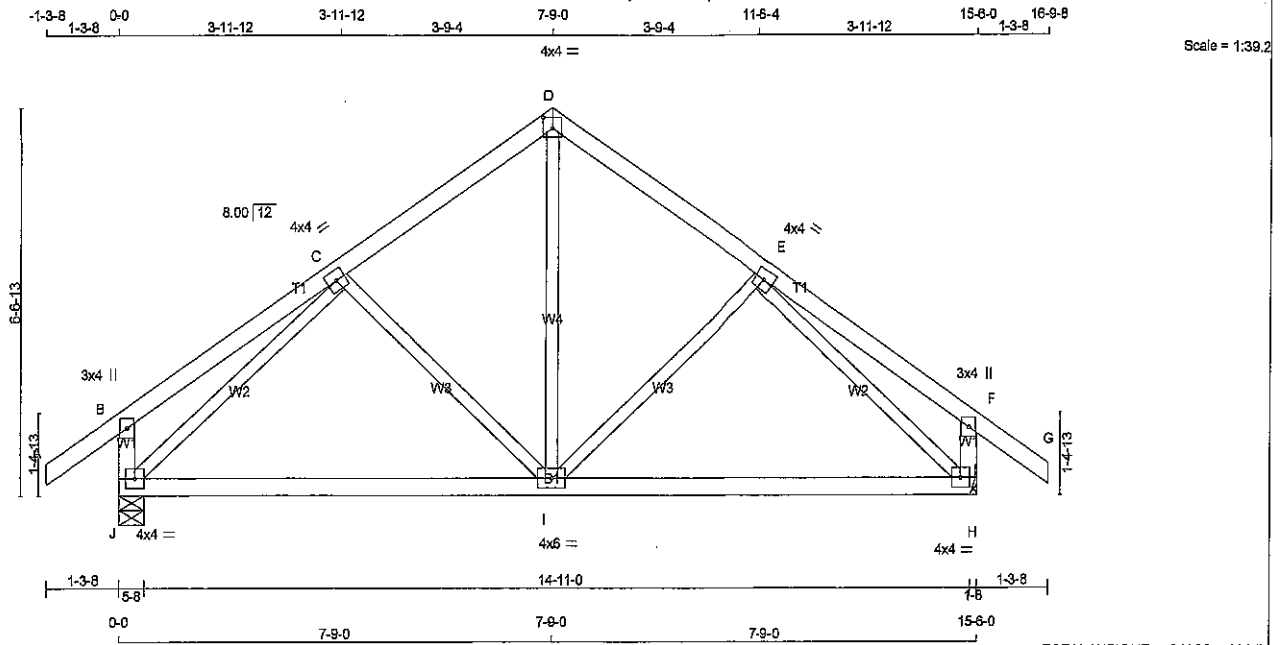
DWG NO. TAM 45645-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
285993	T142	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
J - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	3.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
J	988	0	988	0
H	988	0	988	0

UNFACTORED REACTIONS

JT	1ST CASE	MAX./MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	793	487 / 0	163 / 0	0 / 0	0 / 0	0 / 0	163 / 0	0 / 0
H	793	487 / 0	163 / 0	0 / 0	0 / 0	0 / 0	163 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MAX. FACTORED		FACTORED			MAX. FACTORED				
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX. CS1 (LC)	MEMB.	FORCE (LBS)	MAX. CS1 (LC)		
FR-TO		FROM TO		LENGTH	FR-TO				
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	I-D	0 / 509	0.11 (1)	
B-C	0 / 21	-84.3	-84.3	0.20 (1)	10.00	I-E	-175 / 43	0.08 (1)	
C-D	-708 / 0	-84.3	-84.3	0.16 (1)	8.25	C-I	-175 / 43	0.08 (1)	
D-E	-708 / 0	-84.3	-84.3	0.16 (1)	6.25	J-C	-983 / 0	0.44 (1)	
E-F	0 / 21	-84.3	-84.3	0.20 (1)	10.00	E-H	-983 / 0	0.44 (1)	
F-G	0 / 32	-84.3	-84.3	0.11 (1)	10.00				
J-B	-241 / 0	0.0	0.0	0.02 (1)	7.81				
H-F	-241 / 0	0.0	0.0	0.02 (1)	7.81				
J-I	0 / 700	-28.0	-28.0	0.57 (2)	10.00				
I-H	0 / 700	-28.0	-28.0	0.57 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. GIC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.52")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL) = L/360 (0.52")
CALCULATED VERT. DEFL.(TL) = L/999 (0.18")

CSI: TC=0.20 (E-F:1), BC=0.57 (H-I:2), WB=0.44 (E-H:1), SSI=0.18 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1697 622 2284 1656

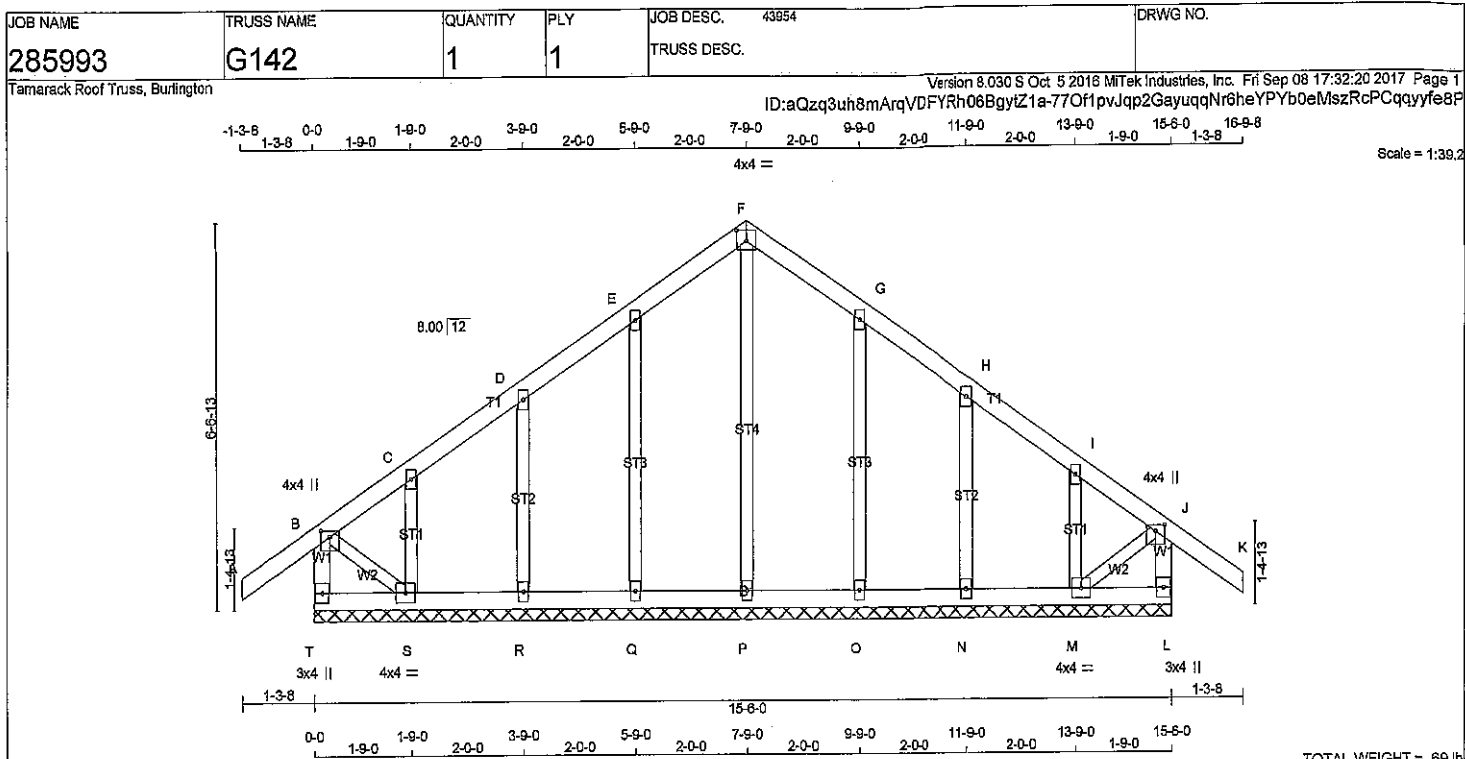
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (D) (INPUT = 0.90)
JSI METAL= 0.34 (C) (INPUT = 1.00)



DRWG NO. TAM 45646-17
STRUCTURAL
COMPONENT ONLY



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - F	2x4	DRY	No.2
F - K	2x4	DRY	No.2
T - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
T - L	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			
GABLE STUDS SPACED AT 2'-0" OC.			

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMVW+p	MT20	4.0	4.0 1.25 2.00
C, D, E, G, H, I	TMVW+p	MT20	4.0	4.0 2.25 2.00
F	TTW+p	MT20	4.0	4.0 1.25 2.00
J	TMVW+p	MT20	4.0	4.0 1.25 2.00
L	BMV1+p	MT20	3.0	4.0
M	BMVW1+t	MT20	4.0	4.0
N, O, P, Q, R	BMV1+w	MT20	2.0	4.0
S	BMVW1+t	MT20	4.0	4.0
T	BMV1+p	MT20	3.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH
FR-TO		FROM TO		FR-TO		FROM TO	
A-B	0/32	-84.3 -84.3	0.11 (1)	10.00	P-F	-129/0	0.09 (1)
B-C	-47/0	-84.3 -84.3	0.11 (1)	6.25	Q-E	-189/0	0.08 (1)
C-D	-14/0	-84.3 -84.3	0.04 (1)	6.25	R-D	-173/0	0.04 (1)
D-E	-13/0	-84.3 -84.3	0.05 (1)	6.25	S-C	-102/0	0.02 (1)
E-F	-20/0	-84.3 -84.3	0.05 (1)	6.25	O-G	-189/0	0.08 (1)
F-G	-20/0	-84.3 -84.3	0.05 (1)	6.25	N-H	-173/0	0.04 (1)
G-H	-13/0	-84.3 -84.3	0.05 (1)	6.25	M-I	-102/0	0.02 (1)
H-I	-14/0	-84.3 -84.3	0.04 (1)	6.25	B-S	0/23	0.01 (1)
I-J	-47/0	-84.3 -84.3	0.11 (1)	8.25	M-J	0/23	0.01 (1)
J-K	0/32	-84.3 -84.3	0.11 (1)	10.00			
T-B	-252/0	0.0 0.0	0.03 (1)	7.81			
L-J	-252/0	0.0 0.0	0.03 (1)	7.81			
T-S	0/0	-28.0 -28.0	0.02 (2)	10.00			
S-R	0/16	-28.0 -28.0	0.02 (2)	10.00			
R-Q	0/12	-28.0 -28.0	0.02 (2)	10.00			
Q-P	0/9	-28.0 -28.0	0.02 (2)	10.00			
P-O	0/9	-28.0 -28.0	0.02 (2)	10.00			
O-N	0/12	-28.0 -28.0	0.02 (2)	10.00			
N-M	0/16	-28.0 -28.0	0.02 (2)	10.00			
M-L	0/0	-28.0 -28.0	0.02 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.8	PSF
	DL = 3.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 46.1	PSF

SPACING = 24.0 IN. GIC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(65 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11 (A-B:1), BC=0.02 (M-N:2), WB=0.09 (F-P:1), SSI=0.08 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	818	354	1667 822 2284 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (J) (INPUT = 0.90)

JSI METAL= 0.05 (E) (INPUT = 1.00)



DWG NO. YAM 45669-17
STRUCTURAL
COMPONENT ONLY

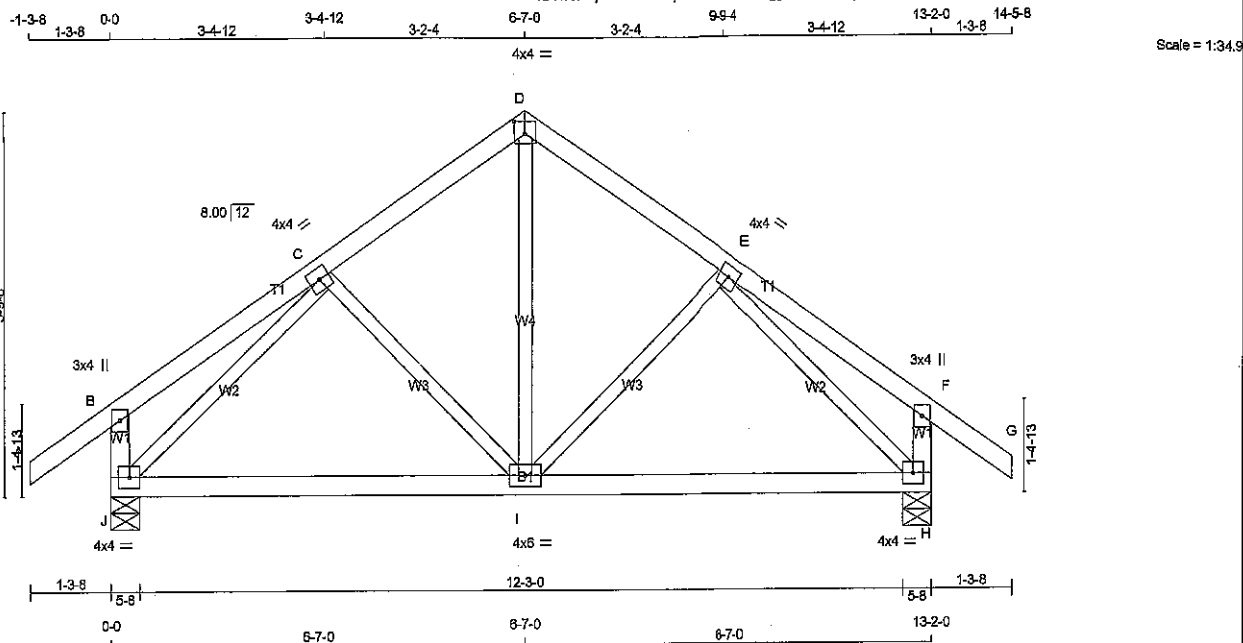
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
285993	T144	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 57 = 115 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	DRY	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
J - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMVW1-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT.	JT	DOWN	JT	IN-SX	JT	IN-SX
J	855	0	855	0	5-8	5-8	5-8
H	855	0	855	0	5-8	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS	JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS
J	COMBINED	SNOW	J	COMBINED	SNOW
J	685	407 / 0	J	685	407 / 0
H	685	407 / 0	H	685	407 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS1 (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 32	-84.3 -84.3	0.11 (1)	I-D	0 / 411	0.09 (1)	
B-C	0 / 17	-84.3 -84.3	0.14 (1)	I-E	-130 / 44	0.05 (1)	
C-D	-583 / 0	-84.3 -84.3	0.11 (1)	C-I	-130 / 44	0.05 (1)	
D-E	-583 / 0	-84.3 -84.3	0.11 (1)	J-C	-795 / 0	0.27 (1)	
E-F	0 / 17	-84.3 -84.3	0.14 (1)	E-H	-795 / 0	0.27 (1)	
F-G	0 / 32	-84.3 -84.3	0.11 (1)				
J-B	-223 / 0	0.0 0.0	0.02 (1)				
H-F	-223 / 0	0.0 0.0	0.02 (1)				
J-I	0 / 563	-28.0 -28.0	0.41 (2)				
I-H	0 / 563	-28.0 -28.0	0.41 (2)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 3.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.44")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL) = L/360 (0.44")
CALCULATED VERT. DEFL.(TL) = L/999 (0.09")

CSI: TC=0.14 (E-F:1), BC=0.41 (I-J:2), WB=0.27 (E-H:1), SSI=0.16 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

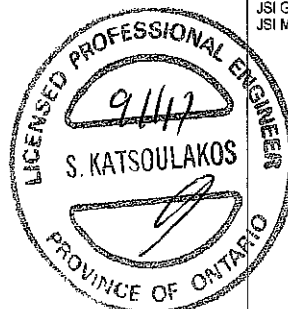
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP(DRY)	SHEAR	SECTION	(PSI)	(PLI)
MT20	618	354	1667	622	2264

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.70 (C) (INPUT = 0.90)
JSI METAL= 0.28 (E) (INPUT = 1.00)

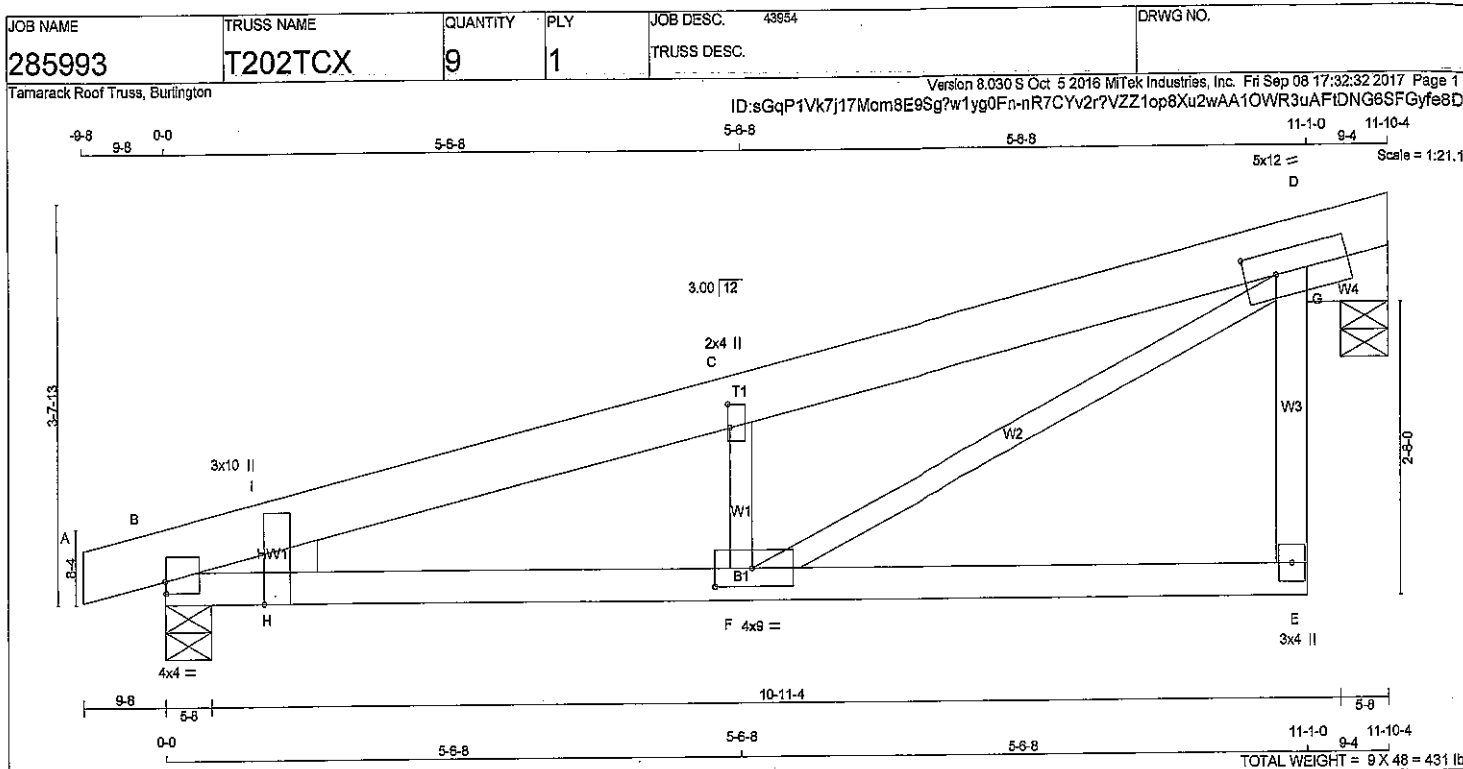


DWONG.TAM 45648-17
STRUCTURAL
COMPONENT ONLY



JSI GRIP= 0.90 (K) (INPUT = 0.90)
JSI METAL= 0.61 (H) (INPUT = 1.00)





LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	4.0	4.0	1.25	
B	WP+I	MT20	3.0	10.0	Edge	11.50
C	TMW+w	MT20	2.0	4.0	2.50	0.25
D	TMVWW1-I	MT20	5.0	12.0	2.50	3.75
E	BMV+p	MT20	3.0	4.0		
F	BMVWW-I	MT20	4.0	9.0	2.00	4.25

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG	HEEL WEDGE
	VERT	HORZ	DOWN	HORZ			
J(D)	593	0	593	0	5-8	5-8	
B	722	0	722	0	5-8	5-8	2x4 L

UNFACTORED REACTIONS

JT	COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
J(D)	487	270 / 0	111 / 0	0 / 0	0 / 0	106 / 0	0 / 0
B	583	340 / 0	122 / 0	0 / 0	0 / 0	121 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J(D), B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO	LENGTH FR-TO	FR-TO			
A-B	-4 / 0	-84.3 -84.3	0.02 (1)	10.00	F-C	-460 / 0	0.07 (1)
B-I	-1371 / 0	-84.3 -84.3	0.04 (1)	6.25	F-D	0 / 1271	0.29 (1)
I-C	-1265 / 0	-84.3 -84.3	0.10 (1)	6.25	H-I	0 / 145	0.00 (1)
C-D	-1258 / 0	-84.3 -84.3	0.19 (1)	6.25	D-J	-828 / 0	0.03 (1)
E-G	0 / 113	0.0 0.0	0.37 (1)	10.00	G-J	0 / 578	0.00 (1)
G-D	0 / 113	0.0 0.0	0.37 (1)	10.00			
B-H	0 / 1231	-28.0 -28.0	0.22 (1)	10.00			
H-F	0 / 1231	-28.0 -28.0	0.32 (2)	10.00			
F-E	0 / 104	-28.0 -28.0	0.19 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, 8BCB 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.37")
CALCULATED VERT. DEFL. (LL) = L/999 (0.06")
ALLOWABLE DEFL. (TL) = L/360 (0.37")
CALCULATED VERT. DEFL. (TL) = L/999 (0.11")

CSI: TC=0.37 (E-G:1), BC=0.32 (F-H:2), WB=0.29 (D-F:1), SSI=0.16 (D-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1658

PLATE PLACEMENT TOL. = 0.250 inches

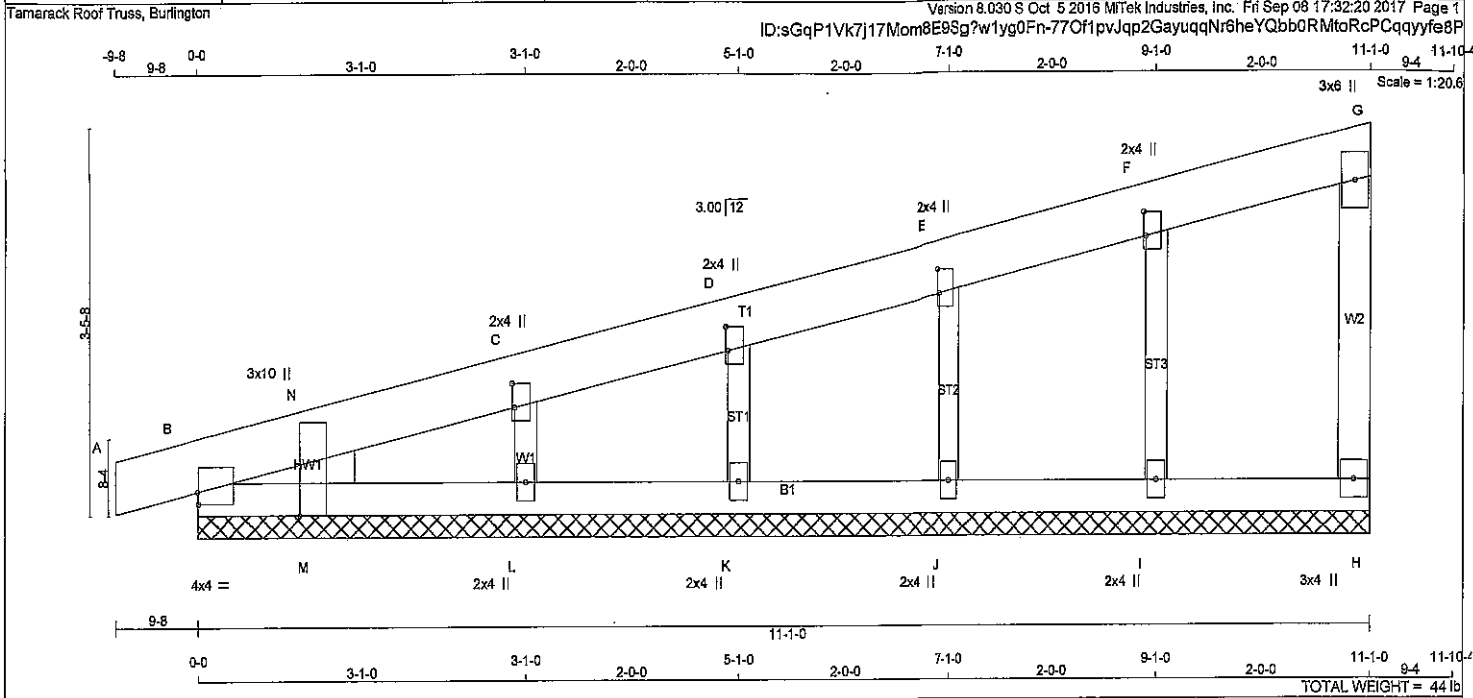
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (F) (INPUT = 0.90)
JSI METAL= 0.44 (B) (INPUT = 1.00)



DRW NO. TAM 45850-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
285993	G202	1	1	TRUSS DESC.		



LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - G	2x8	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
B - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				
GABLE STUDS SPACED AT 2-0-0 OC.				

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMB1-I	MT20	4.0	4.0 1.25
B	WP+I	MT20	3.0	10.0 Edge 11.50
C, D, E, F				
C	TMBV+w	MT20	2.0	4.0 2.50 0.25
G	TMBV+p	MT20	3.0	6.0
H	BMBV1+p	MT20	3.0	4.0
I, J, K, L				
I	BMBV1+w	MT20	2.0	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.									
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.									
HEEL WEDGE 2x4 L BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)									
BRACING									
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.									
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.									
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.									
LOADING									
TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED UNBRACED LENGTH (FT)	
FR-TO		FROM TO			FR-TO				
A-B	-4 / 0	-84.3 -84.3	0.02 (1)	10.00	I-F	-185 / 0	0.03 (1)		
B-N	-57 / 0	-84.3 -84.3	0.04 (1)	6.25	J-E	-171 / 0	0.03 (1)		
N-C	-11 / 4	-84.3 -84.3	0.05 (1)	6.25	K-D	-138 / 0	0.02 (1)		
C-D	-20 / 0	-84.3 -84.3	0.05 (1)	6.25	L-C	-256 / 0	0.04 (1)		
D-E	-8 / 0	-84.3 -84.3	0.02 (1)	10.00	M-N	0 / 63	0.00 (1)		
E-F	-4 / 0	-84.3 -84.3	0.02 (1)	10.00					
F-G	-4 / 0	-84.3 -84.3	0.02 (1)	10.00					
H-G	-72 / 0	0.0 0.0	0.01 (1)	7.81					
B-M	0 / 27	-28.0 -28.0	0.03 (1)	10.00					
M-L	0 / 27	-28.0 -28.0	0.04 (2)	10.00					
L-K	0 / 14	-28.0 -28.0	0.04 (2)	10.00					
K-J	0 / 8	-28.0 -28.0	0.02 (2)	10.00					
J-I	0 / 4	-28.0 -28.0	0.03 (2)	10.00					
I-H	0 / 0	-28.0 -28.0	0.03 (3)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.05 (C-D:1), BC=0.04 (L-M:2), WB=0.04 (C-L:1), SSI=0.09 (C-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1667 822 2284 1856

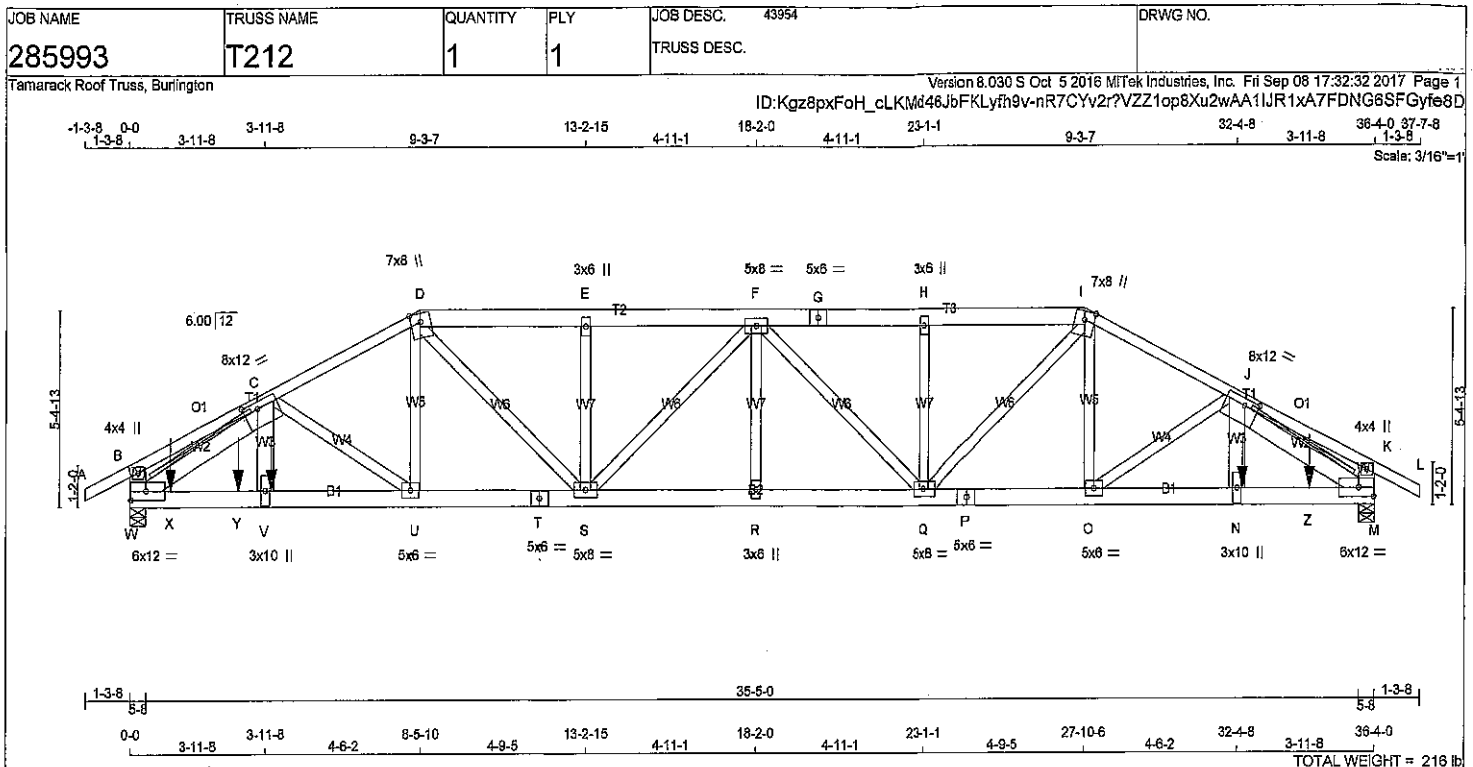
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (C) (INPUT = 0.90)
JSI METAL= 0.05 (L) (INPUT = 1.00)

DWG NO. TAM 45670.17
STRUCTURAL
COMPONENT ONLY

DWG NO. TAM45670.17
STRUCTURAL
COMPONENT ONLY



LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA				
N. L. G. A. RULES												
CHORDS SIZE				LUMBER				SPECIFIED LOADS:				
A - D	2x4	DRY	No.2	SPF	FACTORED	MAXIMUM FACTORED	INPUT	REQD	TOP CH.	LL =	25.6	PSF
D - G	2x6	DRY	No.2	SPF	GROSS REACTION	GROSS REACTION	BRG	BRG	DL =	3.0	PSF	
G - I	2x6	DRY	No.2	SPF	JT VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX		
I - L	2x4	DRY	No.2	SPF	W	4733	0	4733	0	0	5-8	5-8
W - B	2x6	DRY	No.2	SPF	M	4700	0	4700	0	0	5-8	5-8
M - K	2x6	DRY	No.2	SPF								
W - T	2x6	DRY	2100F 1.8E	SPF								
T - P	2x6	DRY	2100F 1.8E	SPF								
P - M	2x6	DRY	2100F 1.8E	SPF								
ALL WEBS EXCEPT				SPF								
V - C	2x6	DRY	No.2	SPF								
N - J	2x6	DRY	No.2	SPF								
W - C	2x6	DRY	No.2	SPF								
J - M	2x6	DRY	No.2	SPF								
DRY: SEASONED LUMBER.												
PLATES (table is in inches)												
JT TYPE	PLATES	W	LEN	Y	X							
B	TMV+p	MT20	4.0	4.0								
C	TMVWW-t	MT20	8.0	12.0	2.25	5.00						
D	TTWW+m	MT20	7.0	8.0	Edge							
E	TMV+w	MT20	3.0	6.0								
F	TMVWW-t	MT20	5.0	8.0								
G	TS-t	MT20	5.0	6.0								
H	TMV+w	MT20	3.0	6.0								
I	TTWW+m	MT20	7.0	8.0	Edge							
J	TMVWW-t	MT20	8.0	12.0	2.25	5.00						
K	TMV+p	MT20	4.0	4.0								
M	BMVW1-t	MT20	6.0	12.0	3.00	5.25						
N	BMV+w	MT20	3.0	10.0								
O	BMVWW-t	MT20	5.0	6.0								
P	BS-t	MT20	5.0	6.0								
Q	BMVWW-t	MT20	5.0	8.0								
R	BMV+w	MT20	3.0	6.0								
S	BMVWW-t	MT20	5.0	8.0								
T	BS-t	MT20	5.0	6.0								
U	BMVWW-t	MT20	5.0	6.0								
V	BMV+w	MT20	3.0	10.0								
W	BMVW1-t	MT20	6.0	12.0	3.00	5.25						
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.												
HANGERS NOTES												
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 74.1 lbs FACTORED DOWN AT 1-2-4, 74.1 lbs FACTORED DOWN AT 3-2-4, 2497.7 lbs FACTORED DOWN AT 4-1-8, AND 2501.2 lbs FACTORED DOWN AT 32-6-8, AND 74.1 lbs FACTORED DOWN AT 34-5-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.												
2x4 DRY SPF No.2 T-BRACE AT C-W, J-M												
FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 8" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.												
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW												
LOADING												
TOTAL LOAD CASES: (4)												
CHORDS												
MEMB. MAX. FACTORED FORCE (LBS)												
FACTORED VERT. LOAD LC1 MAX (PLF)												
MAX. UNBRACED LENGTH FR-TO												
WEBS												
MEMB. MAX. FACTORED FORCE (LBS)												
MAX. FACTORED CSI (LC)												
FR-TO												
A-B												
B-C												
C-D												
D-E												
E-F												
F-G												
G-H												
H-I												
I-J												
J-K												
K-L												
W-B												
M-K												
X-Y												
X-Y												
Y-V												
Y-V												
U-T												
T-S												
S-R												
R-Q												
Q-P												
P-O												
O-N												
N-Z												
Z-M												
UNFACTORED REACTIONS												
1ST LCASE												
MAX/MIN. COMPONENT REACTIONS												
JT COMBINED												
SNOW												
LIVE												
PERM. LIVE												
WIND												
DEAD												
SOIL												
W												
M												
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M												
BRACING												
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.45 FT.												
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.												
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.												
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010												
THIS DESIGN COMPLIES WITH:												
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014												
- CSA 086-09												
- TPIC 2011												
(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD												
ALLOWABLE DEFL.(LL)= L/360 (1.21")												
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.27")												
ALLOWABLE DEFL.(TL)= L/360 (1.21")												
CALCULATED VERT. DEFL.(TL)= L/ 991 (0.44")												
CSI: TC=0.77 (C-D:1), BC=0.44 (V-W:1), WB=0.77 (C-W:1), SS=0.17 (I-J:1)												
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00												
COMPANION LIVE LOAD FACTOR = 0.50												
AUTOSOLVE HELPS OFF												
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.												
NAIL VALUES												
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)												
MAX MIN MAX MIN MAX MIN												
MT20 618 354 1667 622 2264 1656												
PLATE PLACEMENT TOL = 0.250 inches												
PLATE ROTATION TOL = 5.0 Deg.												
JSI GRIP= 0.80 (M) (INPUT = 0.90)												
JSI METAL= 0.97 (J) (INPUT = 1.00)												
DWG NO. TAM 45651-17												
STRUCTURAL												
COMPONENT ONLY												
CONTINUED ON PAGE 2												

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
285993	T212	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
N	32-6-8	-2501	-2501	---	BACK	VERT	TOTAL
V	4-1-8	-2498	-2498	---	TOP	VERT	TOTAL
X	1-2-4	-42	-74	---	BACK	VERT	TOTAL
Y	3-2-4	-42	-74	---	BACK	VERT	TOTAL
Z	34-5-12	-42	-74	---	BACK	VERT	TOTAL



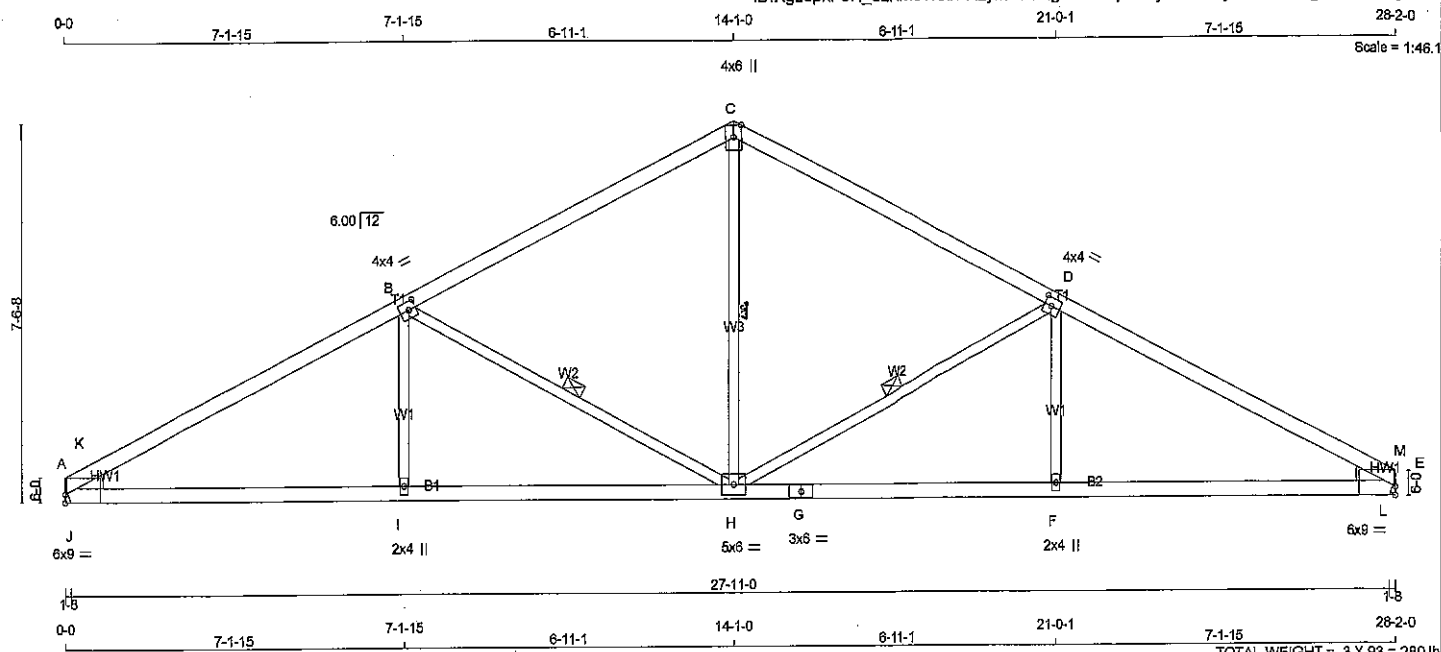
8624
DWG NO. TAM 45651-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43854	DRWG NO.
285993	T215	3	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 3 X 93 = 280 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
A - G	2x4	DRY	No.2	SPF	
G - E	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT		REQD	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	IN-SX	BRG	WEDGE
A	1581	0	1581	0	0				
E	1581	0	1581	0	0				

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
A	1298	721 / 0	298 / 0	0 / 0	0 / 0	282 / 0	0 / 0	0 / 0
E	1298	721 / 0	298 / 0	0 / 0	0 / 0	282 / 0	0 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.62 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-H, B-H. *C-H*

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-K	-3072 / 0	-84.3 -84.3	0.40 (1)	3.62	H-C	0 / 1091	0.25 (1)
K-B	-2842 / 0	-84.3 -84.3	0.64 (1)	3.70	H-D	-928 / 0	0.39 (1)
B-C	-1796 / 0	-84.3 -84.3	0.57 (1)	4.39	F-D	0 / 342	0.08 (3)
C-D	-1766 / 0	-84.3 -84.3	0.57 (1)	4.39	B-H	-928 / 0	0.39 (1)
D-M	-2842 / 0	-84.3 -84.3	0.64 (1)	3.70	I-B	0 / 342	0.08 (3)
M-E	-3072 / 0	-84.3 -84.3	0.40 (1)	3.62	J-K	0 / 1153	0.00 (1)
					L-M	0 / 1153	0.00 (1)
A-J	0 / 2371	-28.0 -28.0	0.52 (1)	10.00			
J-I	0 / 2371	-28.0 -28.0	0.53 (2)	10.00			
I-H	0 / 2371	-28.0 -28.0	0.55 (2)	10.00			
H-G	0 / 2371	-28.0 -28.0	0.55 (2)	10.00			
G-F	0 / 2371	-28.0 -28.0	0.55 (2)	10.00			
F-L	0 / 2371	-28.0 -28.0	0.53 (2)	10.00			
L-E	0 / 2371	-28.0 -28.0	0.52 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
DL = 3.0	PSF	
BOT CH.	LL = 10.5	PSF
DL = 7.0	PSF	
TOTAL LOAD = 45.1	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/380$ (0.94")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.15")
ALLOWABLE DEFL.(TL) = $L/380$ (0.94")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.25")

CSI: TC=0.84 (B-K:1), BC=0.55 (F-H:2), WB=0.39 (D-H:1), SS=0.77 (E-L:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1667
	822	2284	1655

PLATE PLACEMENT TOL. = 0.250 inches

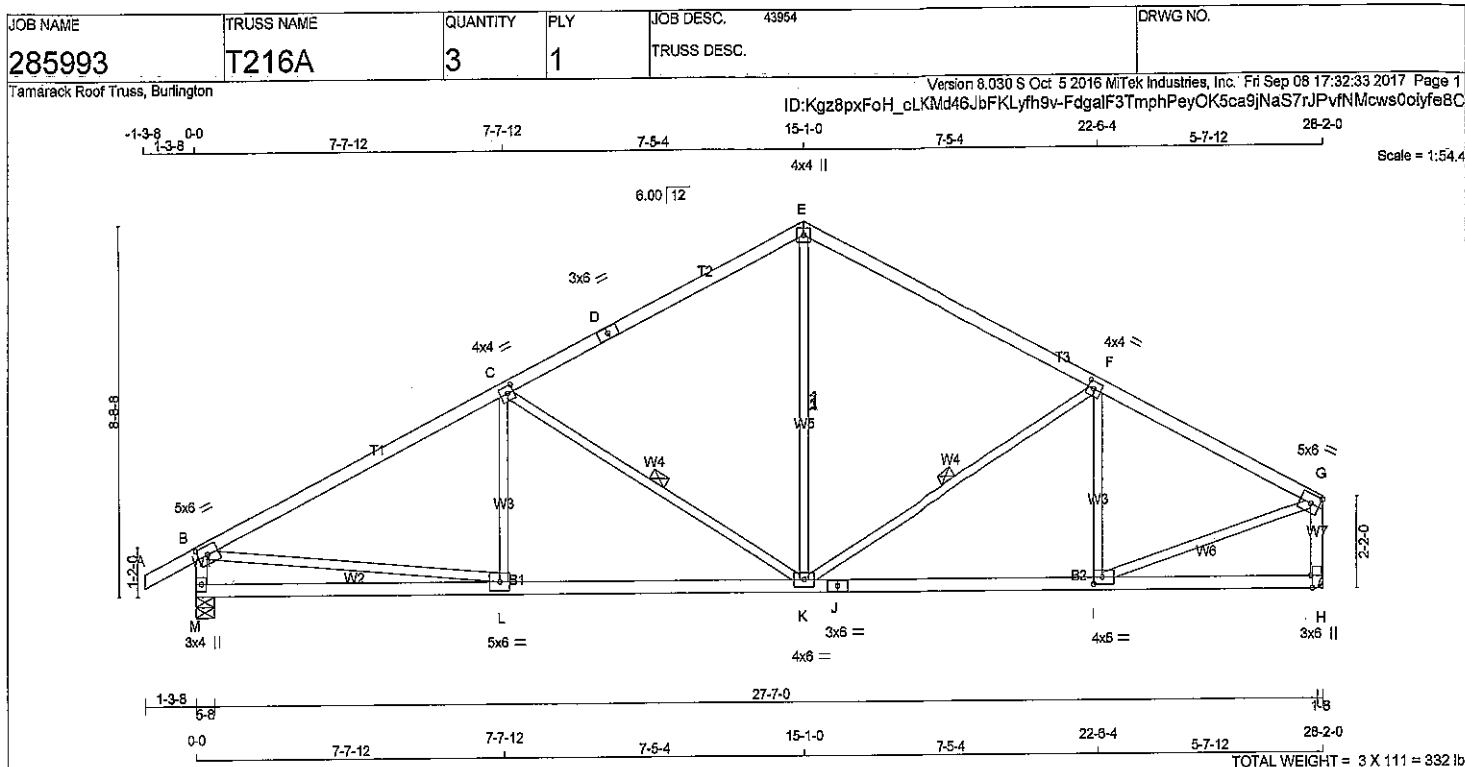
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (E) (INPUT = 0.90)

JSI METAL= 0.59 (G) (INPUT = 1.00)



DRWG NO. TAM 4565417
STRUCTURAL
COMPONENT ONLY



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2 SPF
D - E	2x4	DRY	No.2 SPF
E - G	2x4	DRY	No.2 SPF
M - B	2x4	DRY	No.2 SPF
H - G	2x4	DRY	No.2 SPF
M - J	2x4	DRY	No.2 SPF
J - H	2x4	DRY	No.2 SPF
ALL WEBS EXCEPT	2x3	DRY	No.2 SPF
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-t	MT20	5.0	6.0	2.50 2.75
C	TMVW-t	MT20	4.0	4.0	2.00 1.75
D	TS-t	MT20	3.0	6.0	
E	TTW+p	MT20	4.0	4.0	
F	TMVW-t	MT20	4.0	4.0	2.00 1.75
G	TMVW-t	MT20	5.0	6.0	Edge
H	BMV1+t	MT20	3.0	6.0	Edge 0.50
I	BMVW-t	MT20	4.0	6.0	2.00 2.50
J	BS-t	MT20	3.0	6.0	
K	BMVW-t	MT20	4.0	6.0	
L	BMVW-t	MT20	5.0	6.0	
M	BMV1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	COMBINED	GROSS REACTION	DOWN	GROSS REACTION	UP/LIFT	BRG	IN-SX	BRG	IN-SX
M	1895	0	1895	0	0	5-8	5-8		
H	1581	0	1581	0	0				

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1378	790 / 0	295 / 0	0 / 0	0 / 0	280 / 0	0 / 0
H	1298	721 / 0	295 / 0	0 / 0	0 / 0	282 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.80 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-K, F-K *2x4*

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH FR-TO	LC1 MAX CSI (LC)
FR-TO							
A-B	0 / 28	-84.3	-84.3 0.11 (1)	L-C	0 / 302	0.07 (3)	
B-C	-2243 / 0	-84.3	-84.3 0.83 (1)	C-K	-830 / 0	0.44 (1)	
C-D	-1528 / 0	-84.3	-84.3 0.70 (1)	K-E	0 / 847	0.19 (1)	
D-E	-1528 / 0	-84.3	-84.3 0.70 (1)	K-F	-368 / 0	0.20 (1)	
E-F	-1519 / 0	-84.3	-84.3 0.63 (1)	I-F	-342 / 87	0.13 (1)	
F-G	-1803 / 0	-84.3	-84.3 0.54 (1)	B-L	0 / 2049	0.46 (1)	
M-B	-1807 / 0	0.0	0.0 0.16 (1)	I-G	0 / 1732	0.39 (1)	
H-G	-1521 / 0	0.0	0.0 0.18 (1)				
M-L	0 / 0	-28.0	-28.0 0.39 (3)				
L-K	0 / 2036	-28.0	-28.0 0.62 (2)				
K-J	0 / 1648	-28.0	-28.0 0.49 (2)				
J-I	0 / 1646	-28.0	-28.0 0.49 (2)				
I-H	0 / 0	-28.0	-28.0 0.28 (3)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.94")
CALCULATED VERT. DEFL.(LL) = L/999 (0.18")
ALLOWABLE DEFL.(TL) = L/360 (0.94")
CALCULATED VERT. DEFL.(TL) = L/999 (0.28")

CSI: TC=0.83 (B-C:1), BC=0.62 (K-L:2), WB=0.46 (B-L:1), SSI=0.28 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1667 822 2284 1656

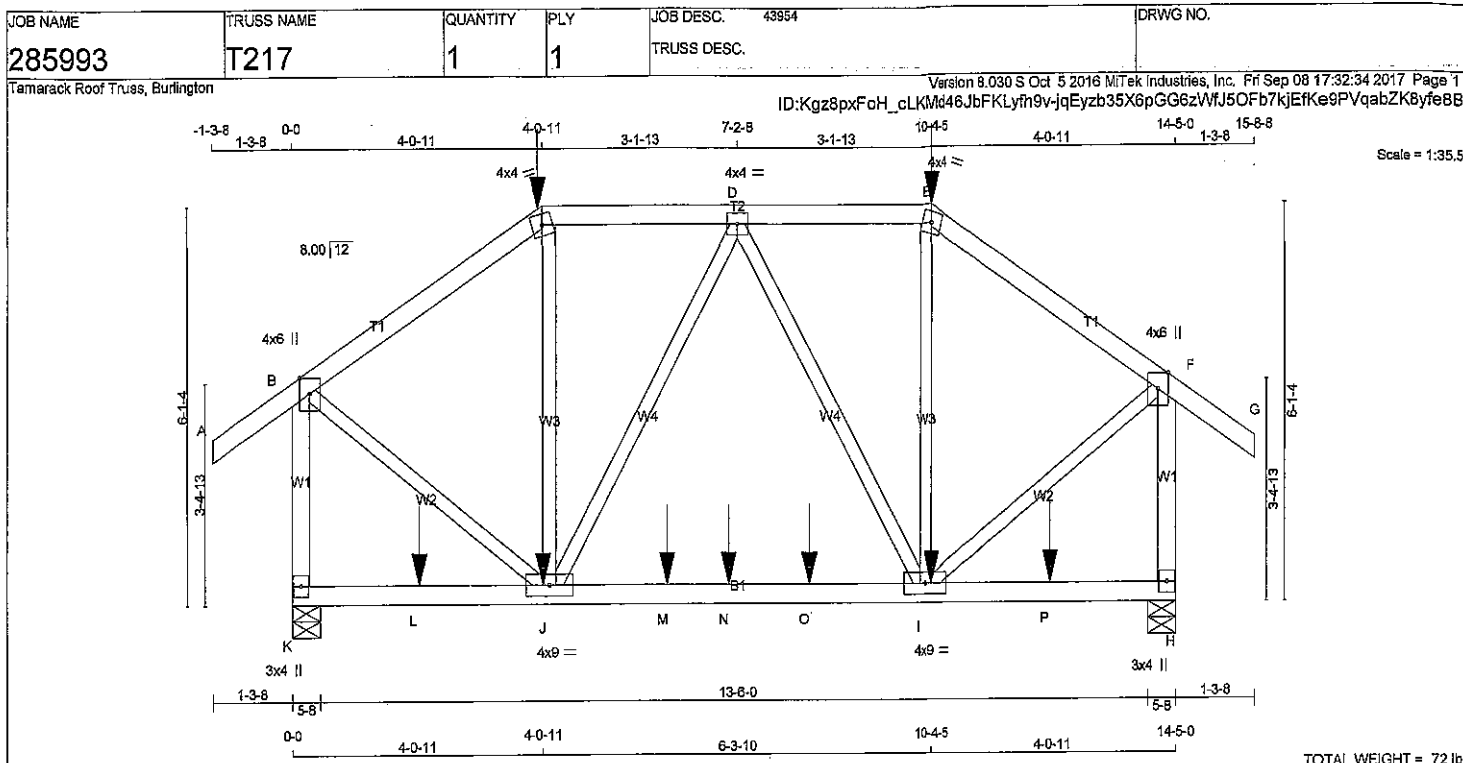
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.89 (L) (INPUT = 0.80)
JSI METAL= 0.61 (B) (INPUT = 1.00)



DWG NO. TAM 45255-17
STRUCTURAL
COMPONENT ONLY



CHORDS	SIZE	DRY	NO.2	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE PLATES	4.0	6.0	Edge	
B TMVW+p MT20	4.0	4.0		
C TTW-m MT20	4.0	4.0		
D TMWW-t MT20	4.0	4.0		
E TTW-m MT20	4.0	4.0		
F TMVW+p MT20	4.0	8.0	Edge	
H BMV1+p MT20	3.0	4.0		
I BMWW-t MT20	4.0	9.0		
J BMWW-t MT20	4.0	9.0		
K BMV1+p MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 244.5 lbs FACTORED DOWN AT 4-0-11, AND 244.5 lbs FACTORED DOWN AT 10-4-5 ON TOP CHORD, AND 110.7 lbs FACTORED DOWN AT 2-0-12, 138.0 lbs FACTORED DOWN AT 4-0-12, 138.0 lbs FACTORED DOWN AT 8-0-12, 138.0 lbs FACTORED DOWN AT 7-0-12, 138.0 lbs FACTORED DOWN AT 8-4-4, AND 138.0 lbs FACTORED DOWN AT 10-4-4, AND 110.7 lbs FACTORED DOWN AT 12-4-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	UPLIFT	IN-SX
K	1627	0	0	5-8
H	1624	0	0	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
K	1246	836 / 0	187 / 0	0 / 0	0 / 0	223 / 0
H	1244	835 / 0	186 / 0	0 / 0	0 / 0	223 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.62 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL FITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
FR-TO					FR-TO			
A-B	0 / 32	-84.3	-84.3	0.12 (1)	10.00	J-C	0 / 170	0.04 (3)
B-C	-1107 / 0	-84.3	-84.3	0.29 (1)	5.62	J-D	-176 / 0	0.14 (1)
C-D	-921 / 0	-84.3	-84.3	0.16 (1)	6.22	D-I	-182 / 0	0.15 (1)
D-E	-918 / 0	-84.3	-84.3	0.16 (1)	6.23	I-E	0 / 169	0.04 (3)
E-F	-1104 / 0	-84.3	-84.3	0.29 (1)	5.63	B-J	0 / 1144	0.28 (1)
F-G	0 / 32	-84.3	-84.3	0.12 (1)	10.00	I-F	0 / 1141	0.28 (1)
K-B	-1583 / 0	0.0	0.0	0.32 (1)	6.52			
H-F	-1580 / 0	0.0	0.0	0.32 (1)	6.52			
K-L	0 / 0	-28.0	-28.0	0.44 (1)	10.00			
L-J	0 / 0	-28.0	-28.0	0.44 (1)	10.00			
J-M	0 / 1003	-28.0	-28.0	0.64 (1)	10.00			
M-N	0 / 1003	-28.0	-28.0	0.64 (1)	10.00			
N-O	0 / 1003	-28.0	-28.0	0.64 (1)	10.00			
O-I	0 / 1003	-28.0	-28.0	0.64 (1)	10.00			
I-P	0 / 0	-28.0	-28.0	0.44 (1)	10.00			
P-H	0 / 0	-28.0	-28.0	0.44 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	4-0-11	-23	-25	---	FRONT	VERT	DEAD
C	4-0-11	-222	-222	---	FRONT	VERT	SNOW
E	10-4-5	-23	-25	---	FRONT	VERT	DEAD
E	10-4-5	-222	-222	---	FRONT	VERT	SNOW
I	10-4-4	-138	-138	---	FRONT	VERT	TOTAL
J	4-0-12	-138	-138	---	FRONT	VERT	TOTAL
L	2-0-12	-111	-111	---	FRONT	VERT	TOTAL
M	6-0-12	-138	-138	---	FRONT	VERT	TOTAL
N	7-0-12	-138	-138	---	FRONT	VERT	TOTAL
O	8-4-4	-138	-138	---	FRONT	VERT	TOTAL
P	12-4-4	-111	-111	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL = 25.8	PSF
	DL = 3.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.48")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.13")
ALLOWABLE DEFL.(TL) = L/360 (0.48")
CALCULATED VERT. DEFL.(TL) = 1/840 (0.21")

CSI: TC=0.32 (B-K:1), BC=0.64 (I-J:1), WB=0.28 (B-J:1), SSI=0.25 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PLI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 822 2284 1656

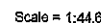
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (B) (INPUT = 0.90)
JSI METAL= 0.28 (B) (INPUT = 1.00)

DRWG NO. TAM 43656-17
STRUCTURAL
COMPONENT ONLY

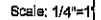




TOTAL WEIGHT = 81 lb

DRY: SEASONED LUMBER.

DWG NO. TAM 2576-18
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = $3 \times 102 = 305 \text{ lb}$

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1687.1 lbs FACTORED DOWN AT 5-4, 1687.2 lbs FACTORED DOWN AT 2-5-4, 1687.2 lbs FACTORED DOWN AT 4-5-4, 1687.2 lbs FACTORED DOWN AT 6-5-4, 1687.2 lbs FACTORED DOWN AT 8-5-4, 1552.9 lbs FACTORED DOWN AT 10-5-4, AND 1552.9 lbs FACTORED DOWN AT 12-5-4, AND 1360.2 lbs FACTORED DOWN AT 13-8-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
I	5-4	-1675	-1675	—	BACK	VERT	TOTAL
J	2-5-4	-1657	-1657	—	BACK	VERT	TOTAL
K	4-5-4	-1657	-1657	—	BACK	VERT	TOTAL
L	6-5-4	-1657	-1657	—	BACK	VERT	TOTAL
M	8-5-4	-1657	-1657	—	BACK	VERT	TOTAL
N	10-5-4	-1553	-1553	—	BACK	VERT	TOTAL
O	12-5-4	-1553	-1553	—	BACK	VERT	TOTAL
P	13-8-12	-1550	-1550	—	BACK	VERT	TOTAL

JSI GRIP= 0.84 (B) (INPUT = 0.90)
JSI METAL= 0.54 (B) (INPUT = 1.00)



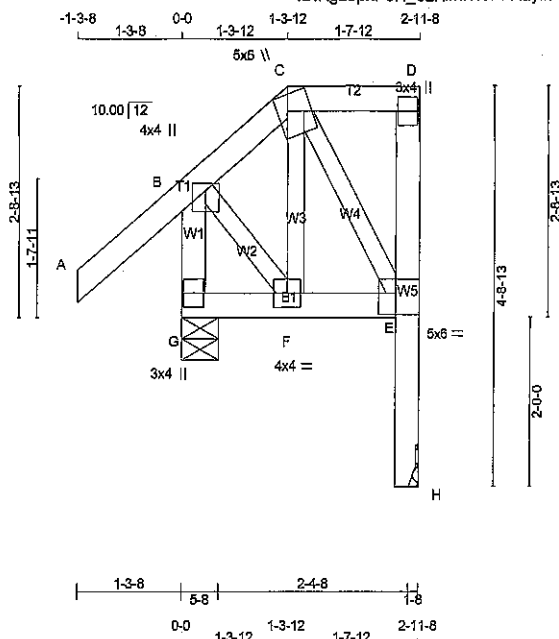
DWONG, TAN 257718
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
285993	T220	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1:27.4

TOTAL WEIGHT = 2 X 21 = 41 lb

LUMBER					
N. L. G. A. RULES					
CHORDS		SIZE	LUMBER		DESCR.
A - C	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
H - D	2x4	DRY	No.2	SPF	
G - B	2x4	DRY	No.2	SPF	
G - E	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT		2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMV+p	MT20	3.0	4.0		
E	BVMW+l	MT20	5.0	6.0	3.00	2.50
F	BMVW+l	MT20	4.0	4.0		
G	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
H	139	0	139	0	0	0
G	310	0	310	0	0	0

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	118	59 / -17	31 / 0	0 / 0	0 / 0	28 / 0	0 / 0
G	234	153 / 0	31 / 0	0 / 0	0 / 0	40 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. UNBRACED	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED		
FR-TO		FROM TO		LENGTH	FR-TO				
A-B	0 / 37	-84.3	-84.3	0.12 (1)	10.00	F-C	0 / 81	0.01 (3)	
B-C	-83 / 0	-84.3	-84.3	0.11 (1)	6.25	C-E	-62 / 11	0.01 (2)	
C-D	0 / 0	-84.3	-84.3	0.04 (1)	10.00	B-F	-11 / 45	0.01 (1)	
H-E	-139 / 0	0.0	0.0	0.02 (1)	7.81				
E-D	-69 / 0	0.0	0.0	0.01 (1)	7.81				
G-B	-292 / 0	0.0	0.0	0.03 (1)	7.81				
G-F	0 / 0	-28.0	-28.0	0.01 (3)	10.00				
F-E	-6 / 35	-28.0	-28.0	0.02 (2)	10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.12 (A-B:1), BC=0.02 (E-F:2), WB=0.01 (C-F:3), SS=0.07 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

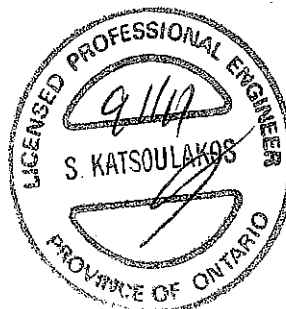
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1887 522 2284 1858

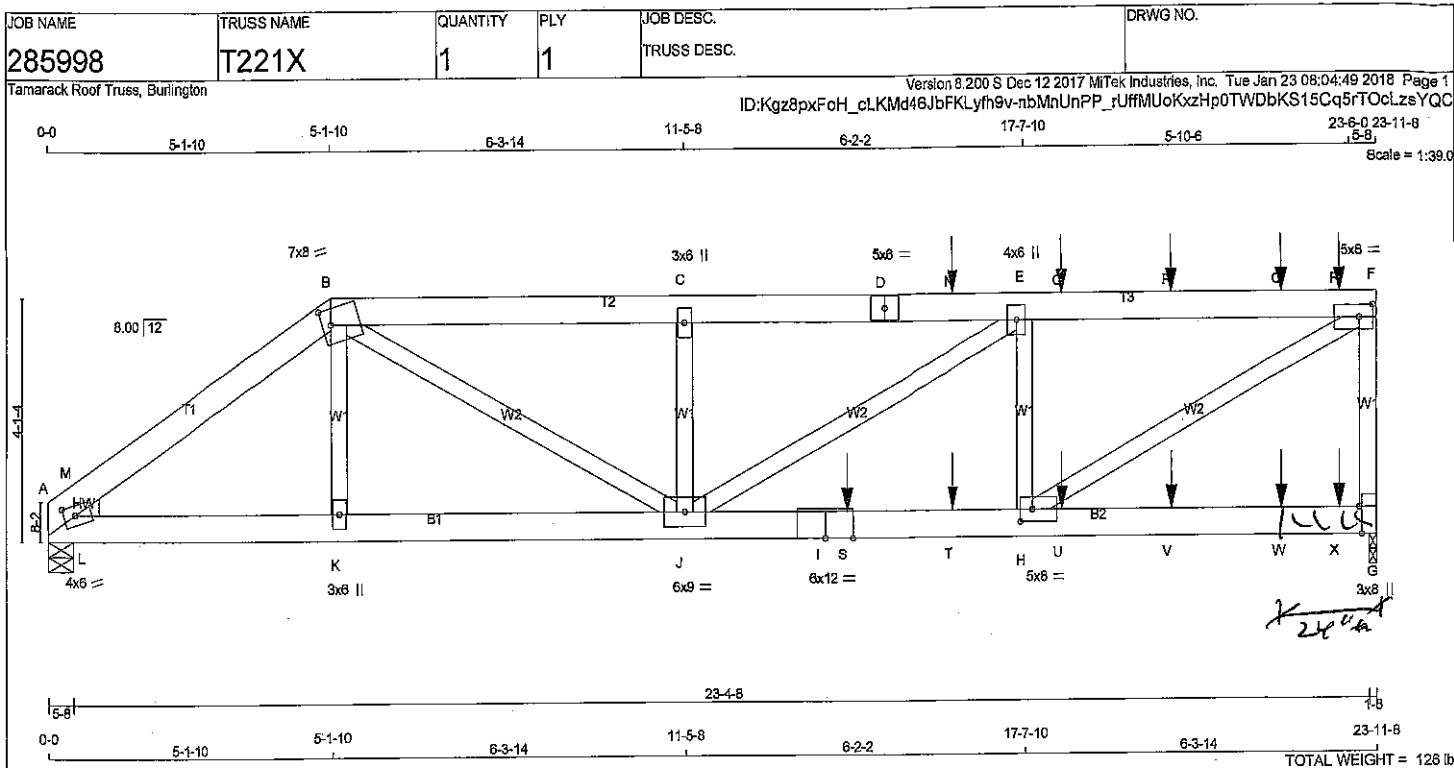
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)



DRWG NO. TAM 45659-17
STRUCTURAL
COMPONENT ONLY



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESC.
A - B	2x6	DRY No.2	SPF
B - D	2x6	DRY No.2	SPF
D - F	2x6	DRY No.2	SPF
G - F	2x4	DRY No.2	SPF
A - I	2x6	DRY 2100F 1.8E	SPF
I - G	2x6	DRY 2100F 1.8E	SPF

ALL WEBS 2x4 DRY No.2 SPF
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBH1-m	MT20	4.0	6.0	2.00	2.50
B	TTWW-m	MT20	7.0	6.0	3.25	1.75
C	TMWW-w	MT20	3.0	6.0		
D	TS-t	MT20	5.0	6.0		
E	TMWW-w	MT20	4.0	6.0		
F	TMWW-t	MT20	5.0	6.0	2.50	2.75
G	BMV1-p	MT20	3.0	6.0	Edge	0.50
H	BMWW-t	MT20	5.0	6.0	2.50	2.75
I	BS-t	MT20	6.0	12.0		
J	BMWW-w	MT20	6.0	6.0		
K	BMW-w	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 101.4 lbs FACTORED DOWN AT 16-3-12, 101.4 lbs FACTORED DOWN AT 18-3-12, 101.4 lbs FACTORED DOWN AT 20-3-12, AND 101.4 lbs FACTORED DOWN AT 22-3-12, AND 116.3 lbs FACTORED DOWN AT 23-3-12 ON TOP CHORD, AND 89.0 lbs FACTORED DOWN AT 14-4-8, 69.9 lbs FACTORED DOWN AT 16-3-12, 69.9 lbs FACTORED DOWN AT 18-3-12, 69.9 lbs FACTORED DOWN AT 20-3-12, AND 89.9 lbs FACTORED DOWN AT 22-3-12, AND 78.6 lbs FACTORED DOWN AT 23-3-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	VERT	HORZ	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG	HEEL WEDGE
G	2492	0	2492	0	1-8	
A	1815	0	1815	0	5-8	2x4 L

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	2052	1129 / 0	475 / 0	0 / 0	0 / 0	449 / 0	0 / 0
A	1499	826 / 0	338 / 0	0 / 0	0 / 0	323 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, A

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
A-M	-2789 / 0	-84.3 -84.3	0.14 (1)	4.86	K-B	0 / 225	0.04 (3)
M-B	-2612 / 0	-84.3 -84.3	0.15 (1)	4.99	B-J	0 / 1988	0.35 (1)
B-C	-3884 / 0	-84.3 -84.3	0.35 (1)	4.05	J-C	-535 / 0	0.09 (1)
C-D	-3864 / 0	-84.3 -84.3	0.46 (1)	3.91	J-E	0 / 542	0.10 (1)
D-N	-3864 / 0	-84.3 -84.3	0.46 (1)	3.91	H-E	-1176 / 0	0.20 (1)
N-E	-3864 / 0	-84.3 -84.3	0.46 (1)	3.91	H-F	0 / 3922	0.69 (1)
E-O	-3398 / 0	-84.3 -84.3	0.48 (1)	4.10	L-M	0 / 227	0.00 (1)
O-P	-3398 / 0	-84.3 -84.3	0.48 (1)	4.10			
P-Q	-3398 / 0	-84.3 -84.3	0.48 (1)	4.10			
Q-R	-3398 / 0	-84.3 -84.3	0.48 (1)	4.10			
R-F	-3398 / 0	-84.3 -84.3	0.48 (1)	4.10			
G-F	-2387 / 0	0.0	0.0	5.48			
A-L	0 / 2154	-28.0	-28.0	0.11 (1)	10.00		
L-K	0 / 2154	-28.0	-28.0	0.15 (1)	10.00		
K-J	0 / 2157	-28.0	-28.0	0.28 (1)	10.00		
J-I	0 / 3398	-28.0	-28.0	0.61 (1)	10.00		
I-S	0 / 3398	-28.0	-28.0	0.61 (1)	10.00		
S-T	0 / 3398	-28.0	-28.0	0.61 (1)	10.00		
T-H	0 / 3398	-28.0	-28.0	0.61 (1)	10.00		
H-U	0 / 0	-28.0	-28.0	0.21 (2)	10.00		
U-V	0 / 0	-28.0	-28.0	0.21 (2)	10.00		
V-W	0 / 0	-28.0	-28.0	0.21 (2)	10.00		
W-X	0 / 0	-28.0	-28.0	0.21 (2)	10.00		
X-G	0 / 0	-28.0	-28.0	0.21 (2)	10.00		

1-7418

S. KATSOUKLAKIS

1-7418

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TOTAL
N	16-3-12	-101	-101		BACK	VERT	TOTAL
O	18-3-12	-101	-101		BACK	VERT	TOTAL
P	20-3-12	-101	-101		BACK	VERT	TOTAL
Q	22-3-12	-101	-101		BACK	VERT	TOTAL
R	23-3-12	-116	-116		BACK	VERT	TOTAL
S	14-4-8	-890	-890		BACK	VERT	TOTAL
T	16-3-12	-40	-70		BACK	VERT	TOTAL
U	18-3-12	-40	-70		BACK	VERT	TOTAL
V	20-3-12	-40	-70		BACK	VERT	TOTAL
W	22-3-12	-40	-70		BACK	VERT	TOTAL
X	23-3-12	-45	-79		BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.80")
CALCULATED VERT. DEFL.(LL) = L/999 (0.19")
ALLOWABLE DEFL.(TL) = L/360 (0.80")
CALCULATED VERT. DEFL.(TL) = L/943 (0.30")

CSI: TC=0.55/1.00 (F-G:1), BC=0.61/1.00 (H-J:1), WB=0.69/1.00 (F-H:1), SS=0.37/1.00 (H-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (F) (INPUT = 0.90)
JSI METAL= 0.98 (I) (INPUT = 1.00)

DWG NO. TAM 5060-38
STRUCTURAL
COMPONENT ONLY

SCAB JOINT # G-8 WITH 2 X 6 #2
SPF 24" LONG 1 SIDE(S) USING 2
ROW(S) 3/4" ARDOX SPIRAL COMMON WIRE
NAILS AT 2" O/C STAGGERED (18 NAILS/SCAB).
REQUIRED FOR 1/2" BRG PROVISION.

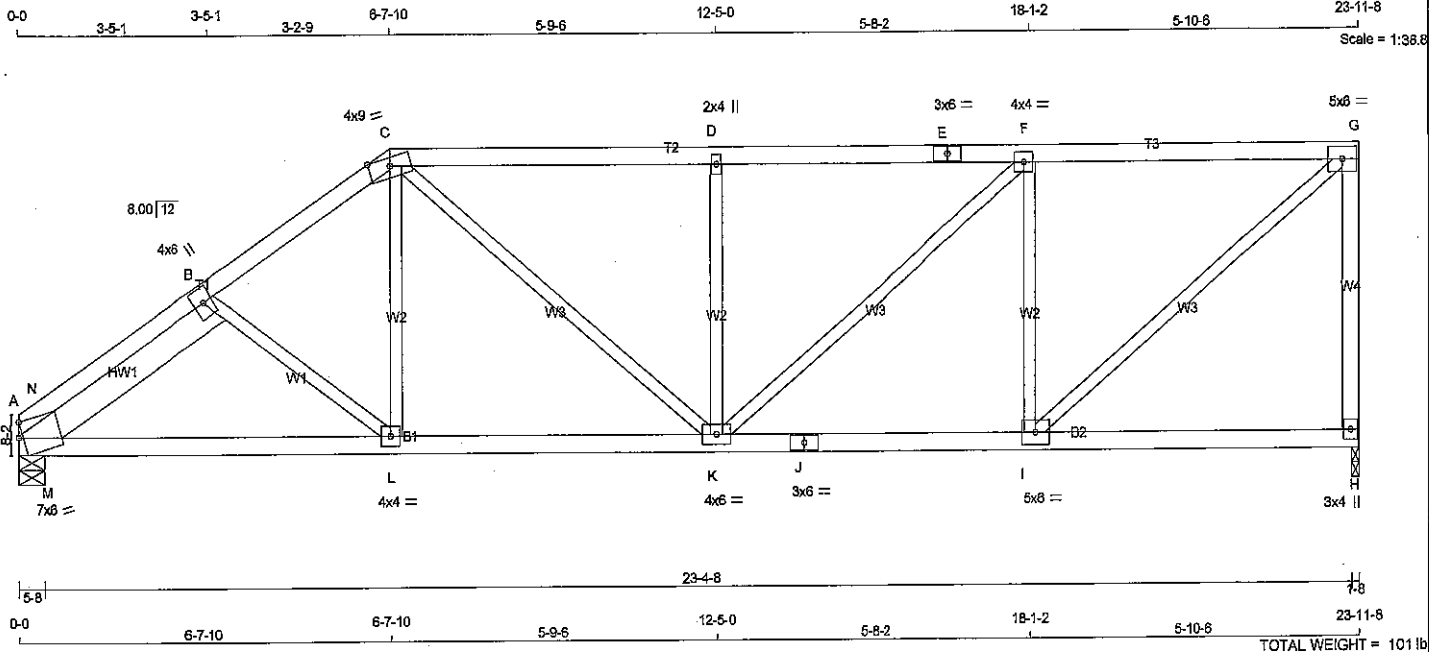


JOB NAME 285998	TRUSS NAME T222X	QUANTITY 1	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.200 S Dec 12 2017 MitTek Industries, Inc. Thu Jan 11 11:25:41 2018 Page 1

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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
H - G	2x4	DRY	No.2	SPF	
A - J	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	

REINFORCING MEMBERS	HW1	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBMW1-m	MT20	7.0	8.0	3.00	Edge
B	TMBWW-H	MT20	4.0	6.0		
C	TTWW-m	MT20	4.0	9.0		Edge
D	TMW-w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMWW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	5.0	8.0		
J	BS-t	MT20	3.0	8.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMWW-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
A	1345	0	1345	0	5-8
H	1345	0	1345	0	1-8

UNFACTORED REACTIONS

JT	1ST CASE	MAX	MIN	COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
A	1104	613 / 0	252 / 0	0 / 0	0 / 0	240 / 0	0 / 0	0 / 0
H	1104	613 / 0	252 / 0	0 / 0	0 / 0	240 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.53 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MEMB.		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	MEMB.	FORCE (LBS)	MAX. UNBRACED LENGTH	MEMB.	FORCE (LBS)	MAX. FACTORED (LC)
FR-TO		FROM	TO	FR-TO			FR-TO		
A-N	-1757 / 0	-84.3	-84.3	0.04 (2)	5.08	B-L	-179 / 0	0.08 (1)	
N-B	-1283 / 0	-84.3	-84.3	0.13 (1)	5.55	L-C	0 / 350	0.08 (2)	
B-C	-1652 / 0	-84.3	-84.3	0.15 (1)	5.02	I-G	0 / 1699	0.38 (1)	
C-D	-1737 / 0	-84.3	-84.3	0.45 (1)	4.55	C-K	0 / 482	0.11 (1)	
D-E	-1737 / 0	-84.3	-84.3	0.47 (1)	4.53	I-F	-894 / 0	0.34 (1)	
E-F	-1737 / 0	-84.3	-84.3	0.47 (1)	4.53	K-D	-522 / 0	0.20 (1)	
F-G	-1313 / 0	-84.3	-84.3	0.44 (1)	5.09	K-F	0 / 555	0.12 (1)	
H-G	-1278 / 0	0.0	0.0	0.56 (1)	7.13	M-N	0 / 549	0.00 (1)	
						M-B	-573 / 0	0.08 (1)	
A-M	0 / 1080	-28.0	-28.0	0.29 (2)	10.00				
M-L	0 / 1500	-28.0	-28.0	0.38 (2)	10.00				
L-K	0 / 1366	-28.0	-28.0	0.38 (2)	10.00				
K-J	0 / 1313	-28.0	-28.0	0.38 (2)	10.00				
J-I	0 / 1313	-28.0	-28.0	0.38 (2)	10.00				
I-H	0 / 0	-28.0	-28.0	0.23 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 3.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.80")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (0.80")
CALCULATED VERT. DEFL.(TL) = L/999 (0.13")

CSI: TC=0.56/1.00 (G-H:1), BC=0.38/1.00 (I-K:2),
WB=0.38/1.00 (G-I:1), SS=0.23/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

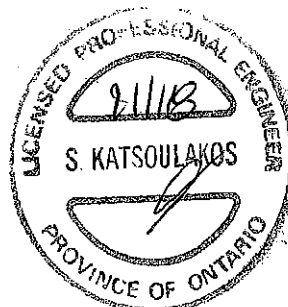
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

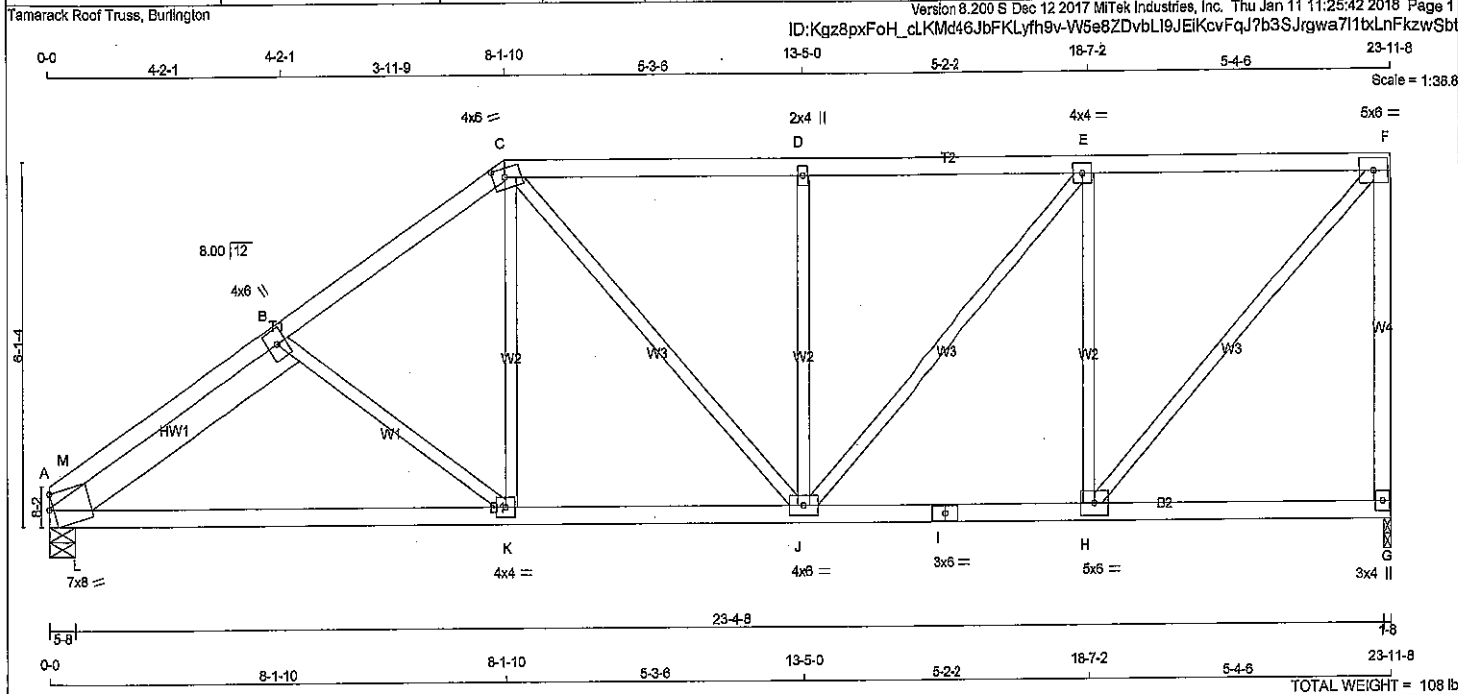
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (C) (INPUT = 0.90)
JSI METAL= 0.39 (J) (INPUT = 1.00)



DWG NO. YAM 2569-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285998	T223X	1	1	TRUSS DESC.	



LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES				
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
A - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF

REINFORCING MEMBERS	HW1	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
A - TMBMW1-m	MT20	7.0	8.0	3.00	Edge		
B - TMBMW1-t	MT20	4.0	6.0				
C - TMBMW1-m	MT20	4.0	6.0	1.75	2.50		
D - TMBMW1-t	MT20	2.0	4.0				
E - TMBMW1-t	MT20	4.0	4.0				
F - TMBMW1-t	MT20	5.0	6.0				
G - TMBMW1-t	MT20	3.0	4.0				
H - TMBMW1-t	MT20	5.0	6.0				
I - TMBMW1-t	MT20	3.0	6.0				
J - TMBMW1-t	MT20	4.0	6.0				
K - TMBMW1-t	MT20	4.0	4.0				

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	UPLIFT	IN-SX
A	1345	0	5-8	5-8
G	1345	0	1-8	1-8

UNFACTORED REACTIONS							
	1ST CASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	1104	613 / 0	252 / 0	0 / 0	0 / 0	240 / 0	0
G	1104	613 / 0	252 / 0	0 / 0	0 / 0	240 / 0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

CHORDS	MEMB.	MAX. FACTORED	FORCE (LBS)	VERT. LOAD	LC1	MAX	CS1 (LC)	MAX	UNBRAC	LENGTH	FR-TO	MEMB.	MAX. FACTORED	FORCE (LBS)	MAX	CS1 (LC)
FR-TO																
A - M	-1890 / 0	-84.3	-84.3	0.06 (3)	4.88	B - K	-285 / 0	0.13 (1)								
M - B	-1392 / 0	-84.3	-84.3	0.22 (1)	5.26	K - C	0 / 420	0.05 (2)								
B - C	-1537 / 0	-84.3	-84.3	0.23 (1)	5.07	H - F	0 / 1499	0.34 (1)								
C - D	-1409 / 0	-84.3	-84.3	0.35 (1)	5.08	C - J	0 / 209	0.05 (1)								
D - E	-1409 / 0	-84.3	-84.3	0.36 (1)	5.05	H - E	-930 / 0	0.54 (1)								
E - F	-1016 / 0	-84.3	-84.3	0.35 (1)	5.74	J - D	-476 / 0	0.28 (1)								
G - F	-1285 / 0	0.0	0.0	0.93 (1)	7.11	J - E	0 / 590	0.13 (1)								
						L - M	0 / 603	0.00 (1)								
						L - B	-416 / 82	0.08 (1)								
A - L	0 / 1174	-28.0	-28.0	0.44 (2)	10.00											
L - K	0 / 1487	-28.0	-28.0	0.45 (2)	10.00											
K - J	0 / 1268	-28.0	-28.0	0.42 (2)	10.00											
J - I	0 / 1016	-28.0	-28.0	0.32 (2)	10.00											
I - H	0 / 1016	-28.0	-28.0	0.32 (2)	10.00											
H - G	0 / 0	-28.0	-28.0	0.20 (3)	10.00											

DESIGN CRITERIA

SPECIFIED LOADS:					
TOP CH.	LL	=	25.6	PSF	
	DL	=	3.0	PSF	
BOT CH.	LL	=	10.5	PSF	
	DL	=	7.0	PSF	
TOTAL LOAD		=	46.1	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.80")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.10")
ALLOWABLE DEFL.(TL) = $L/360$ (0.80")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.17")

CSI: TC=0.93/1.00 (F-G:1), BC=0.45/1.00 (K-L:2),
WB=0.54/1.00 (E-H:1), SS=0.30/1.00 (A-L:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

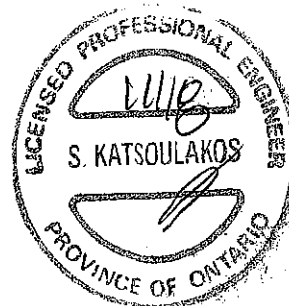
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP(DRY)		SHEAR		SECTION
	{PSI}		{PLI}		{PLI}
	MAX	MIN	MAX	MIN	MAX MIN
MT20	618	354	1867	822	2284 1656

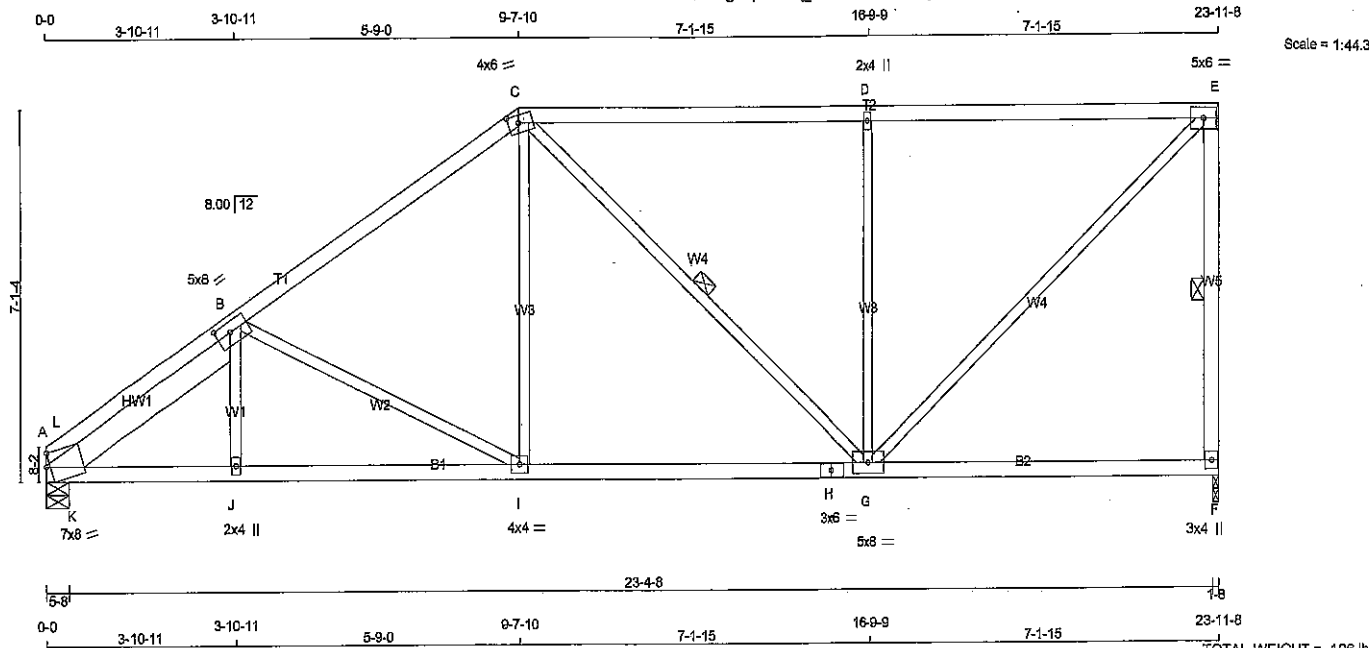
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (C) (INPUT = 0.90)
JSI METAL= 0.34 (I) (INPUT = 1.00)



DRWG NO. TAM 2570-18
STRUCTURAL
COMPONENT ONLY



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - F	2x4	DRY	No.2
REINFORCING MEMBERS			
HW1	2x6	DRY	No.2
ALL WEBS 2x3 DRY, SEASONED LUMBER.			
	2x3	DRY	No.2

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMBMW1-m	MT20	7.0	8.0	3.00 Edge
B	TMBMW1-t	MT20	5.0	8.0	2.25 3.50
C	TTWW-m	MT20	4.0	8.0	1.75 2.50
D	TMW-w	MT20	2.0	4.0	
E	TMW-t	MT20	5.0	6.0	
F	BMV1-p	MT20	3.0	4.0	
G	BMVW-t	MT20	5.0	8.0	
H	BS-t	MT20	3.0	6.0	
I	BMVW-t	MT20	4.0	4.0	
J	BMV-w	MT20	2.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
A	1345	0	1345	0	5-8
F	1345	0	1345	0	1-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	1104	613 / 0	252 / 0	0 / 0	0 / 0	240 / 0	0 / 0
F	1104	613 / 0	252 / 0	0 / 0	0 / 0	240 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.54 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-F, C-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	MEMB.
FR-TO		FROM TO		FR-TO			
A-L	-1587 / 0	-84.3 -84.3	0.05 (1)	5.20	J-B	0 / 245	0.05 (2)
L-B	-1101 / 0	-84.3 -84.3	0.38 (1)	5.52	B-I	-488 / 0	0.38 (1)
B-C	-1406 / 0	-84.3 -84.3	0.43 (1)	5.01	I-C	0 / 463	0.10 (2)
C-D	-1093 / 0	-84.3 -84.3	0.83 (1)	4.54	C-G	-88 / 0	0.05 (2)
D-E	-1093 / 0	-84.3 -84.3	0.83 (1)	4.54	G-D	-748 / 0	0.65 (1)
F-E	-1264 / 0	0.0 0.0	0.29 (1)	5.73	G-E	0 / 1504	0.34 (1)
					K-L	0 / 548	0.00 (1)
A-K	0 / 954	-28.0 -28.0	0.19 (1)	10.00	K-B	-842 / 0	0.15 (1)
K-J	0 / 1583	-28.0 -28.0	0.32 (1)	10.00			
J-I	0 / 1581	-28.0 -28.0	0.40 (2)	10.00			
I-H	0 / 1155	-28.0 -28.0	0.50 (2)	10.00			
H-G	0 / 1155	-28.0 -28.0	0.50 (2)	10.00			
G-F	0 / 0	-28.0 -28.0	0.36 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 25.6	PSF
	DL	= 3.0	PSF
BOT CH.	LL	= 10.5	PSF
	DL	= 7.0	PSF
TOTAL LOAD		= 46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.80")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.11")
ALLOWABLE DEFL.(TL) = $L/360$ (0.80")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.18")

CSI: TC=0.83/1.00 (D-E:1), BC=0.50/1.00 (G-I:2), WS=0.65/1.00 (D-G:1), SS=0.29/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

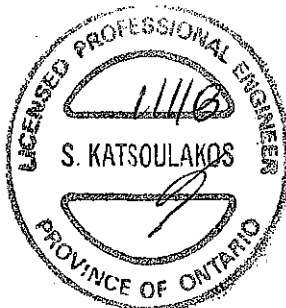
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (H) (INPUT = 0.90)
JSI METAL= 0.52 (H) (INPUT = 1.00)



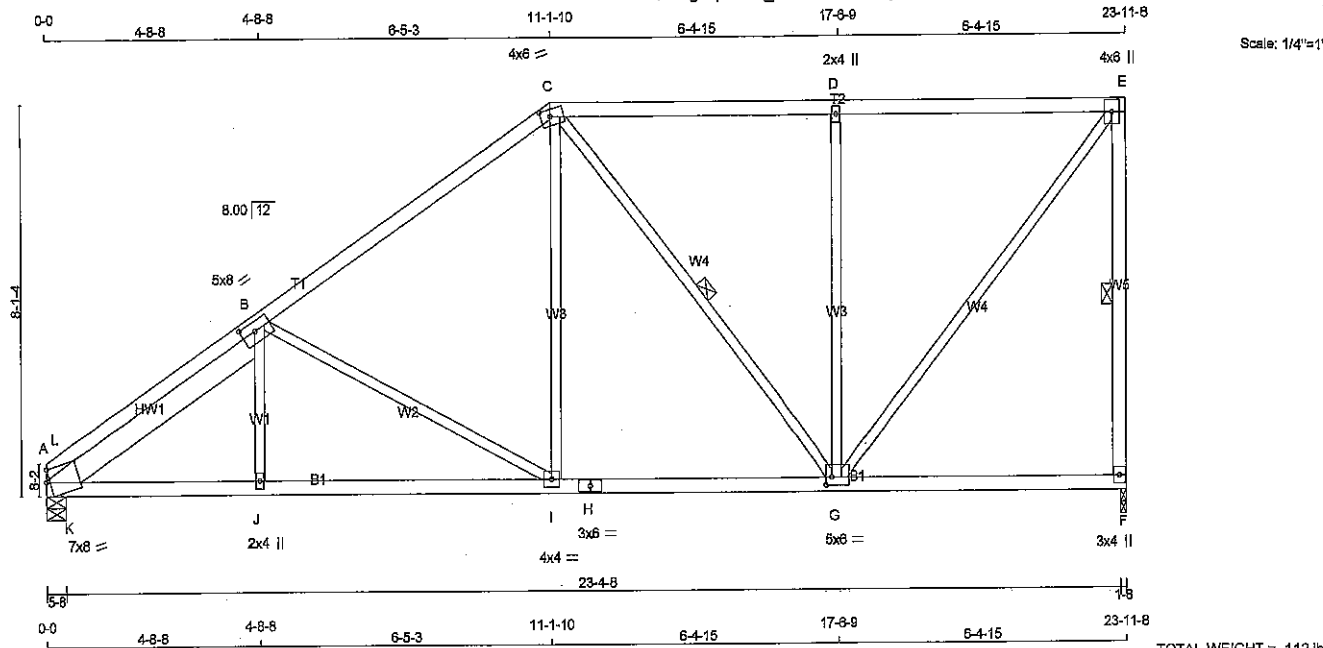
DWG NO. YAM2571-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285998	T225X	1	1	TRUSS DESC.	

Tamareck Roof Truss, Burlington

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TOTAL WEIGHT = 112 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
F - E	2x4	DRY	No.2	SPF	
A - H	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	

REINFORCING MEMBERS	SIZE	LUMBER	DESCR.
HW1	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBMW-1	MT20	7.0	8.0	3.00	Edge
B	TMWWW-t	MT20	5.0	8.0	2.25	3.50
C	TTWW-m	MT20	4.0	8.0	1.75	2.50
D	TMW+w	MT20	2.0	4.0		
E	TMW+w	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	6.0	2.00	1.50
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMW+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
A	1345	0	1345	0
F	1345	0	1345	0

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	1104	613 / 0	252 / 0	0 / 0	0 / 0	240 / 0	0 / 0
F	1104	613 / 0	252 / 0	0 / 0	0 / 0	240 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.04 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-F, C-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	CS1 (LC)	UNBRAC	MEMB.	WEBS	MAX. FACTORED	MAX	CS1 (LC)
		FORCE						LENGTH			FORCE		
FR-TO		(LBS)									(LBS)		
A-L		-1616 / 0	-84.3	-84.3	0.03 (1)	5.19	J-B	0 / 291	0.07 (2)				
L-B		-1146 / 0	-84.3	-84.3	0.48 (1)	5.26	B-I	-801 / 0	0.68 (1)				
B-C		-1272 / 0	-84.3	-84.3	0.55 (1)	5.04	I-C	0 / 510	0.11 (2)				
C-D		-879 / 0	-84.3	-84.3	0.63 (1)	5.48	C-G	-255 / 0	0.16 (1)				
D-E		-879 / 0	-84.3	-84.3	0.63 (1)	5.48	G-D	-869 / 0	0.85 (1)				
F-E		-1271 / 0	0.0	0.0	0.38 (1)	5.71	E-G	0 / 1381	0.31 (1)				
							K-L	0 / 495	0.00 (1)				
A-K		0 / 989	-28.0	-28.0	0.19 (1)	10.00	K-B	-754 / 0	0.20 (1)				
K-J		0 / 1565	-28.0	-28.0	0.37 (2)	10.00							
J-I		0 / 1563	-28.0	-28.0	0.40 (2)	10.00							
I-H		0 / 1041	-28.0	-28.0	0.40 (2)	10.00							
H-G		0 / 1041	-28.0	-28.0	0.40 (2)	10.00							
G-F		0 / 0	-28.0	-28.0	0.28 (3)	10.00							

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 25.6	PSF
	DL	= 3.0	PSF
BOT CH.	LL	= 10.5	PSF
	DL	= 7.0	PSF
TOTAL LOAD		= 46.1	PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.80")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (0.80")
CALCULATED VERT. DEFL.(TL) = L/999 (0.12")

CSI: TC=0.63/1.00 (D-E:1), BC=0.40/1.00 (I-J:2), WB=0.85/1.00 (D-G:1), SS=0.26/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	818	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (G) (INPUT = 0.90)
JSI METAL= 0.35 (H) (INPUT = 1.00)



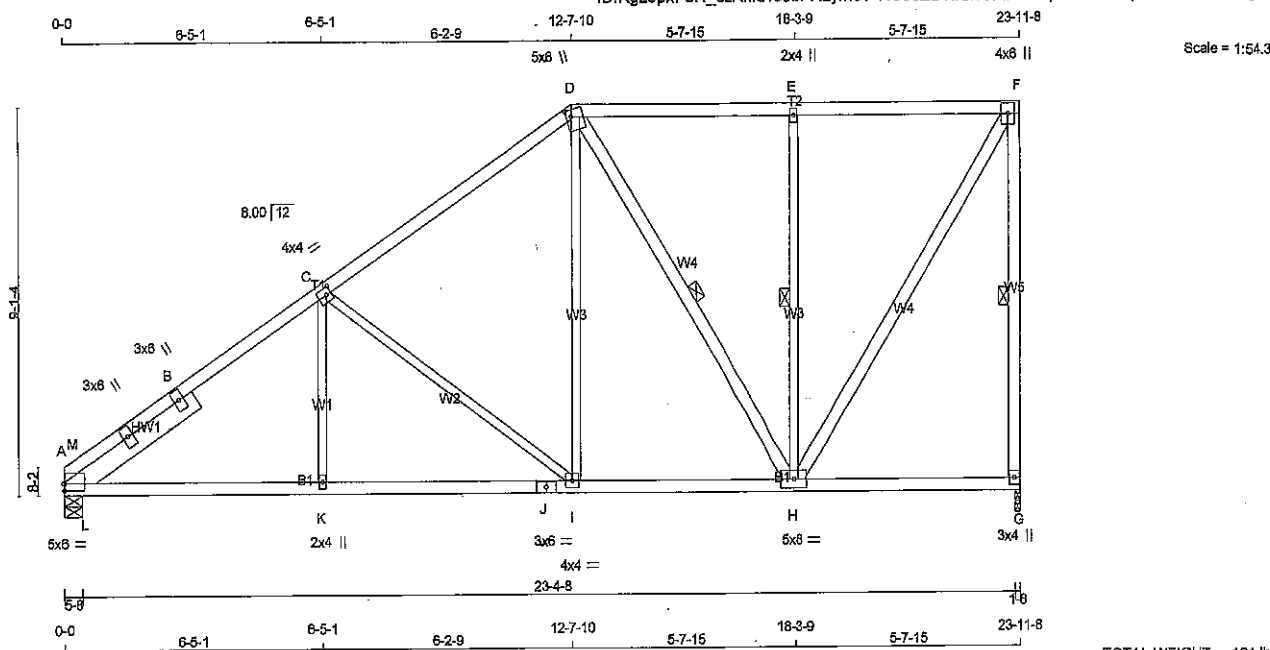
DWG NO. TAM 2572-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285998	T226X	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 121 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
A - J	2x4	DRY	No.2	SPF	
J - G	2x4	DRY	No.2	SPF	

REINFORCING MEMBERS	HW1	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	
D - H	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBMR1-i	MT20	5.0	6.0	2.00	0.25
A	RT-H	MT20	3.0	6.0		
A	RT-H	MT20	3.0	6.0		
C	TMWW-i	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMW+w	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-i	MT20	5.0	6.0		
I	BMWW-i	MT20	4.0	4.0		
J	BS-i	MT20	3.0	6.0		
K	BMW+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
A	1345	0	1345	0
G	1345	0	1345	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX	MIN	COMPONENT REACTIONS
	COMBINED	SNOW	LIVE	PERM.LIVE
A	1104	613 / 0	252 / 0	0 / 0
G	1104	613 / 0	252 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.99 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)
FR-TO									
A-M	-1708 / 0	-84.3	-84.3	0.06 (1)	5.07	K-C	0 / 316	0.07 (2)	
M-B	-1481 / 0	-84.3	-84.3	0.38 (1)	4.99	C-I	-863 / 0	0.84 (1)	
B-C	-1481 / 0	-84.3	-84.3	0.38 (1)	4.99	I-D	0 / 559	0.13 (2)	
C-D	-1128 / 0	-84.3	-84.3	0.41 (1)	5.47	D-H	-394 / 0	0.22 (1)	
D-E	-710 / 0	-84.3	-84.3	0.35 (1)	6.25	H-E	-591 / 0	0.32 (1)	
E-F	-710 / 0	-84.3	-84.3	0.35 (1)	6.25	F-F	0 / 1307	0.21 (1)	
G-F	-1280 / 0	0.0	0.0	0.49 (1)	5.70	L-M	0 / 496	0.00 (1)	
						L-B	-531 / 0	0.06 (1)	
A-L	0 / 1070	-28.0	-28.0	0.25 (2)	10.00				
L-K	0 / 1450	-28.0	-28.0	0.39 (2)	10.00				
K-J	0 / 1450	-28.0	-28.0	0.39 (2)	10.00				
J-I	0 / 1450	-28.0	-28.0	0.39 (2)	10.00				
I-H	0 / 922	-28.0	-28.0	0.32 (2)	10.00				
H-G	0 / 0	-28.0	-28.0	0.21 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD	=	46.1	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.80")
CALCULATED VERT. DEFL.(LL)= L/999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.80")
CALCULATED VERT. DEFL.(TL)= L/999 (0.11")

CSI: TC=0.49/1.00 (F-G:1), BC=0.39/1.00 (K-L:2), WB=0.84/1.00 (C-I:1), SSI=0.23/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1667 822 2284 1666

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

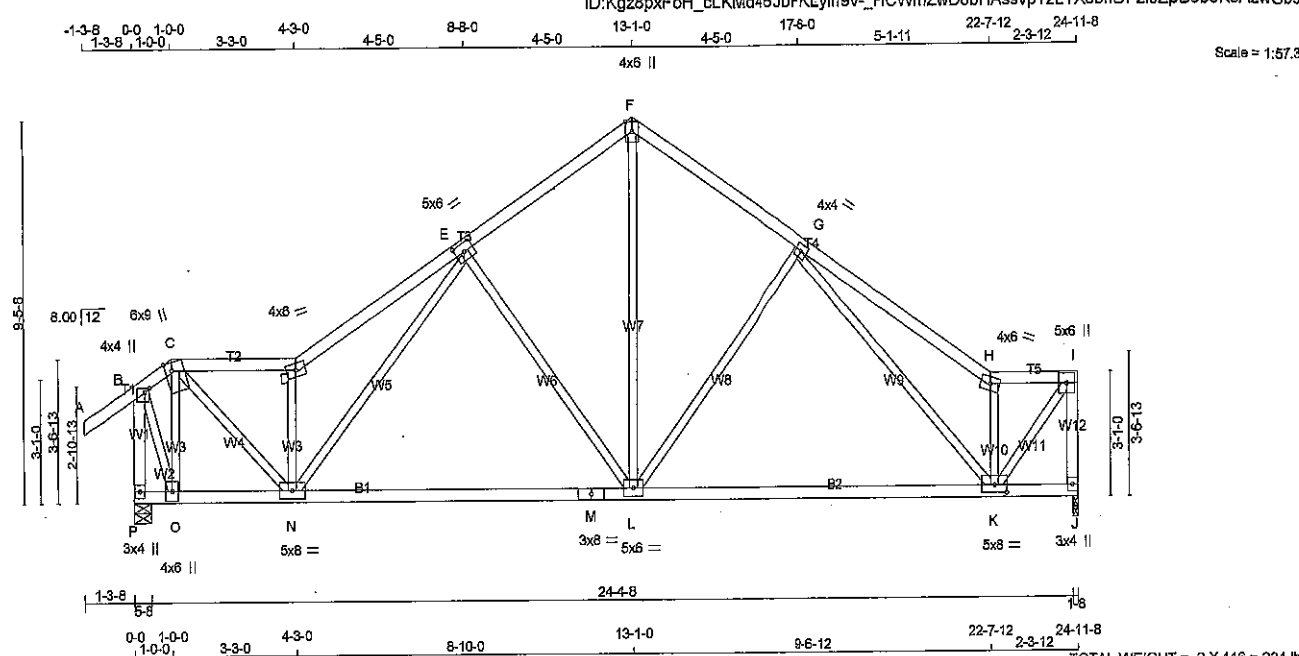
JSI GRIP= 0.73 (F) (INPUT = 0.90)
JSI METAL= 0.43 (A) (INPUT = 1.00)



DWG NO. FAM 2573-10
STRUCTURAL
COMPONENT ONLY

JOB NAME 285998	TRUSS NAME T227S	QUANTITY 2	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington Version 8.200 S Dec 12 2017 MiTek Industries, Inc. Thu Jan 11 11:25:43 2018 Page 1
ID:Kg28pxFoH_cLKMD46JbFKLyfh9v-_HCWmZwD6bHAssvpTzLYXobnDFzIJZpB6b5KoAzwSbs



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - D	2x4	DRY No.2	SPF
D - F	2x4	DRY No.2	SPF
F - H	2x4	DRY No.2	SPF
H - I	2x4	DRY No.2	SPF
J - K	2x4	DRY No.2	SPF
P - B	2x4	DRY No.2	SPF
P - M	2x4	DRY No.2	SPF
M - J	2x4	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	4.0	4.0	1.25	1.50
C	TTWW+m	MT20	6.0	9.0	Edge	2.00
D	TTW-m	MT20	4.0	6.0		
E	TMWW-t	MT20	5.0	6.0	2.50	2.75
F	TTW+p	MT20	4.0	6.0	Edge	
G	TMWW-t	MT20	4.0	4.0		
H	TTW-m	MT20	4.0	6.0		
I	TMWW+p	MT20	5.0	6.0		
J	BMV1+p	MT20	3.0	4.0		
K	BMWWW-t	MT20	5.0	8.0	2.25	4.00
L	BMWWW-t	MT20	5.0	6.0		
M	BS-t	MT20	3.0	8.0		
N	BMWWW-t	MT20	5.0	8.0		
O	BMWWW-t	MT20	4.0	6.0		
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
J	1398	0	1398	0	1-8	1-8
P	1520	0	1520	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE		MAX/MIN COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
J	1148	837 / 0	262 / 0	0 / 0	0 / 0	248 / 0	0 / 0
P	1231	711 / 0	282 / 0	0 / 0	0 / 0	258 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, P

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.74 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		MAX. UNBRACED LENGTH	MEMB.	WEBS	
	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 (PLF)	MAX. CSI (LC)			MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO						FR-TO		
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	N-D	-1289 / 0	0.27 (1)
B-C	-547 / 0	-84.3	-84.3	0.09 (1)	8.25	N-E	0 / 425	0.10 (2)
C-D	-1492 / 0	-84.3	-84.3	0.14 (1)	5.25	E-L	-534 / 0	0.82 (1)
D-E	-1614 / 0	-84.3	-84.3	0.28 (1)	4.74	L-F	0 / 1014	0.23 (1)
E-F	-1198 / 0	-84.3	-84.3	0.23 (1)	5.59	L-G	-378 / 0	0.44 (1)
F-G	-1202 / 0	-84.3	-84.3	0.27 (1)	5.54	G-K	-153 / 108	0.21 (1)
G-H	-1297 / 0	-84.3	-84.3	0.28 (1)	5.37	K-H	-1002 / 0	0.18 (1)
H-I	-1065 / 0	-84.3	-84.3	0.07 (1)	6.04	K-I	0 / 1715	0.39 (1)
I-J	-1435 / 0	0.0	0.0	0.23 (1)	6.82	O-C	-985 / 0	0.20 (1)
P-B	-1491 / 0	0.0	0.0	0.23 (1)	6.71	B-O	0 / 1058	0.24 (1)
						C-N	0 / 1595	0.38 (1)
P-O	0 / 0	-28.0	-28.0	0.05 (3)	10.00			
O-N	0 / 385	-28.0	-28.0	0.33 (2)	10.00			
N-M	0 / 1294	-28.0	-28.0	0.70 (2)	10.00			
M-L	0 / 1294	-28.0	-28.0	0.70 (2)	10.00			
L-K	0 / 1200	-28.0	-28.0	0.68 (2)	10.00			
K-J	0 / 0	-28.0	-28.0	0.40 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH LL = 25.8 PSF
DL = 3.0 PSF
BOT CH LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.83")
CALCULATED VERT. DEFL.(LL) = L/999 (0.20")
ALLOWABLE DEFL.(TL) = L/360 (0.83")
CALCULATED VERT. DEFL.(TL) = L/925 (0.32")

CSI: TC=0.28/1.00 (G+H-1), BC=0.70/1.00 (L-N-2), WB=0.82/1.00 (E-L-1), SS=0.19/1.00 (K-L-2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PS)	SECTION (PL)
MT20	618	354	1567
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL= 0.62 (M) (INPUT = 1.00)



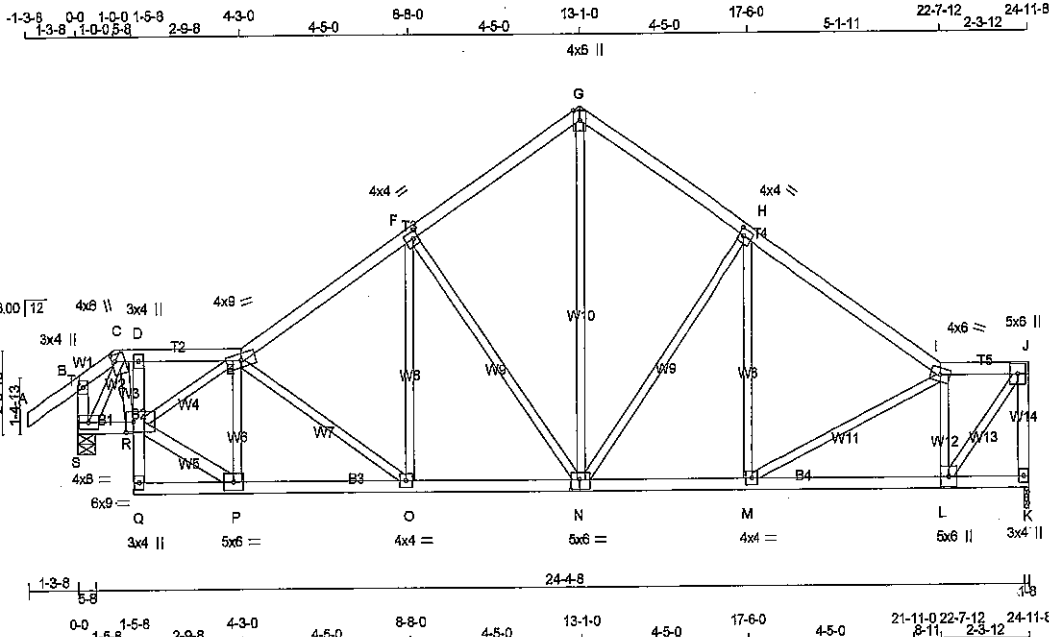
DWG NO. TAM 2574-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285998	T228S	5	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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Scale = 1:56.9

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
G - I	2x4	DRY	No.2	SPF	
I - J	2x4	DRY	No.2	SPF	
J - K	2x4	DRY	No.2	SPF	
K - L	2x4	DRY	No.2	SPF	
L - M	2x4	DRY	No.2	SPF	
M - N	2x4	DRY	No.2	SPF	
N - O	2x4	DRY	No.2	SPF	
O - P	2x4	DRY	No.2	SPF	
P - Q	2x4	DRY	No.2	SPF	
Q - R	2x4	DRY	No.2	SPF	
R - S	2x4	DRY	No.2	SPF	
S - T	2x4	DRY	No.2	SPF	
T - U	2x4	DRY	No.2	SPF	
U - V	2x4	DRY	No.2	SPF	
V - W	2x4	DRY	No.2	SPF	
W - X	2x4	DRY	No.2	SPF	
X - Y	2x4	DRY	No.2	SPF	
Y - Z	2x4	DRY	No.2	SPF	
Z - A	2x4	DRY	No.2	SPF	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
K	1398	0	1398	0
S	1520	0	1520	0

UNFACTORED REACTIONS

	1ST CASE	MAX. MIN. COMPONENT REACTIONS	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED	SNOW					
K	1148	637 / 0	262 / 0	0 / 0	0 / 0	249 / 0	0 / 0
S	1231	711 / 0	262 / 0	0 / 0	0 / 0	258 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, S

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.07 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED HORIZ. LOAD (PLF)	MAX. FACTORED MOMENT (FT-LB)	WEEKS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (PLF)	MAX. FACTORED MOMENT (FT-LB)
FR-TO											
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	0.27 (1)	R-P	0 / 1689	0.27 (1)		
B-C	-41 / 0	-84.3	-84.3	0.08 (1)	6.25	0.08 (1)	R-E	-424 / 0	0.08 (1)		
C-D	-1146 / 0	-84.3	-84.3	0.02 (3)	5.94	0.16 (1)	P-E	-749 / 0	0.16 (1)		
D-E	-1125 / 0	-84.3	-84.3	0.10 (1)	5.89	0.12 (1)	E-O	-240 / 0	0.12 (1)		
E-F	-1547 / 0	-84.3	-84.3	0.24 (1)	5.07	0.08 (2)	O-F	0 / 339	0.08 (2)		
F-G	-1175 / 0	-84.3	-84.3	0.22 (1)	5.64	0.71 (1)	F-N	-508 / 0	0.71 (1)		
G-H	-1175 / 0	-84.3	-84.3	0.26 (1)	5.58	0.22 (1)	N-G	0 / 998	0.22 (1)		
H-I	-1446 / 0	-84.3	-84.3	0.29 (1)	5.13	0.54 (1)	N-H	-466 / 0	0.54 (1)		
I-J	-1022 / 0	-84.3	-84.3	0.07 (1)	8.13	0.05 (3)	M-H	0 / 217	0.05 (3)		
K-J	-1361 / 0	0.0	0.0	0.22 (1)	6.92	0.21 (1)	L-J	-1178 / 0	0.21 (1)		
S-B	-235 / 0	0.0	0.0	0.02 (1)	7.81	0.37 (1)	L-J	0 / 1645	0.37 (1)		
							M-I	0 / 182	0.04 (1)		
S-R	0 / 759	-28.0	-28.0	0.13 (1)	10.00	0.23 (1)	S-C	-1473 / 0	0.23 (1)		
Q-R	0 / 39	0.0	0.0	0.05 (1)	10.00		C-R	0 / 1381	0.31 (1)		
R-D	-165 / 21	0.0	0.0	0.04 (1)	7.81						
Q-P	0 / 24	-28.0	-28.0	0.09 (3)	10.00						
P-O	0 / 1502	-28.0	-28.0	0.29 (1)	10.00						
O-N	0 / 1309	-28.0	-28.0	0.27 (1)	10.00						
N-M	0 / 1226	-28.0	-28.0	0.28 (2)	10.00						
M-L	0 / 1066	-28.0	-28.0	0.28 (2)	10.00						
L-K	0 / 0	-28.0	-28.0	0.11 (3)	10.00						

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 25.6	PSF
	DL	= 3.0	PSF
BOT CH.	LL	= 10.5	PSF
	DL	= 7.0	PSF
TOTAL LOAD		= 46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 8, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 8 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.83")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.07")
ALLOWABLE DEFL. (TL) = $L/360$ (0.83")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.11")

CSI: TC=0.29/1.00 (H-E:1), BC=0.29/1.00 (O-P:1), WB=0.71/1.00 (F-N:1), SS=0.17/1.00 (H-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 822 2284 1656

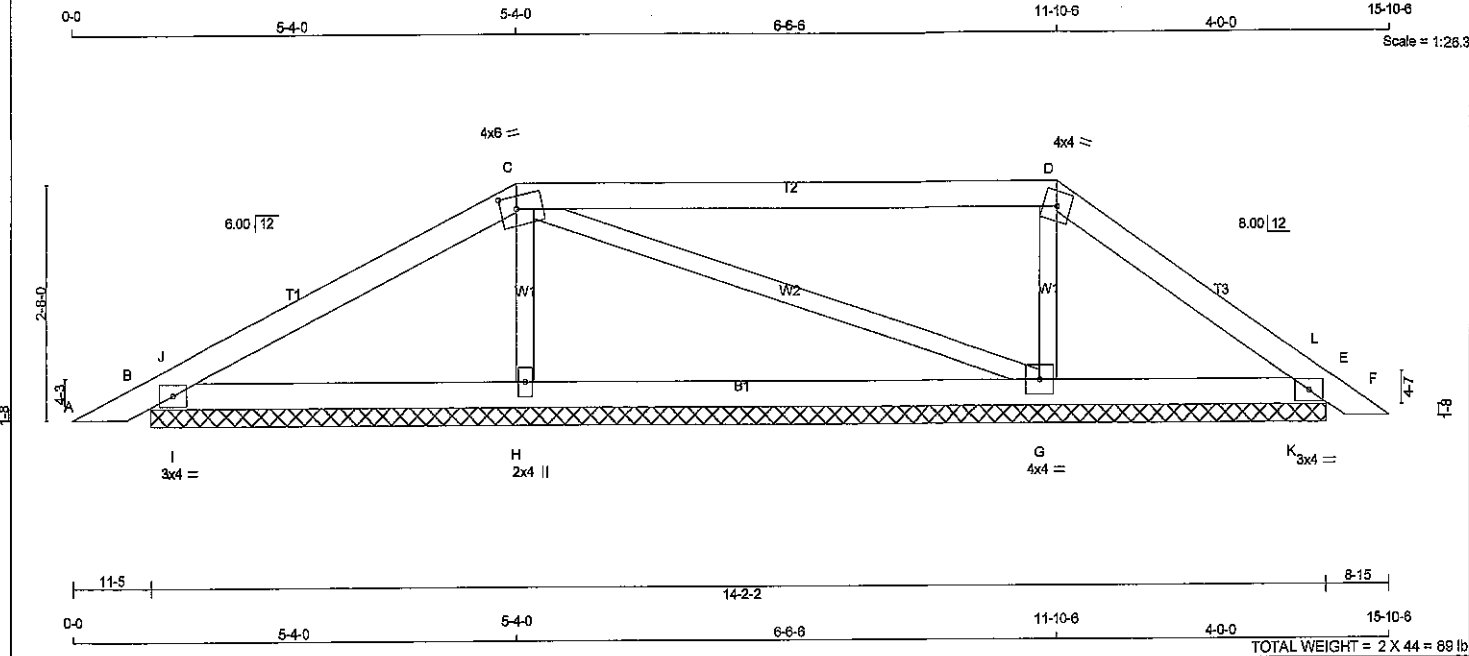
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (N) (INPUT = 0.90)
JSI METAL= 0.42 (I) (INPUT = 1.00)



DRWG NO. TAM 2575-18
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	No.2	DESCR.	SPF
A - C	2x4	DRY	No.2		SPF	
C - D	2x4	DRY	No.2		SPF	
D - F	2x4	DRY	No.2		SPF	
B - E	2x4	DRY	No.2		SPF	

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW-m	MT20	4.0	6.0	1.75	2.25
D	TTWW-m	MT20	4.0	4.0		
E	TMB1-I	MT20	3.0	4.0		
G	BMWW1-t	MT20	4.0	4.0		
H	BMW1-w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
B	349	0	349	0
E	286	0	286	0
H	581	0	581	0
G	528	0	528	0

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED
B	263
E	195
H	490
G	448

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 15	-84.3 -84.3	0.05 (1) 10.00	H-C	-374 / 0	0.06 (1)	
B-J	-85 / 0	-84.3 -84.3	0.08 (1) 6.25	C-G	-27 / 0	0.02 (1)	
J-C	-118 / 0	-84.3 -84.3	0.20 (1) 6.25	G-D	-341 / 0	0.05 (1)	
C-D	-56 / 0	-84.3 -84.3	0.61 (1) 8.25	I-J	-288 / 39	0.00 (1)	
D-L	-93 / 0	-84.3 -84.3	0.11 (1) 8.25	K-L	-232 / 0	0.00 (1)	
L-E	-32 / 0	-84.3 -84.3	0.04 (1) 8.25				
E-F	0 / 14	-84.3 -84.3	0.03 (1) 10.00				
B-I	0 / 100	-28.0 -28.0	0.18 (1) 10.00				
I-H	0 / 100	-28.0 -28.0	0.22 (2) 10.00				
H-G	0 / 82	-28.0 -28.0	0.22 (2) 10.00				
G-K	0 / 72	-28.0 -28.0	0.19 (2) 10.00				
K-E	0 / 72	-28.0 -28.0	0.11 (1) 10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 3.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.61 (C-D:1), BC=0.22 (H-I:2), WB=0.06 (C-H:1), SSI=0.23 (B-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

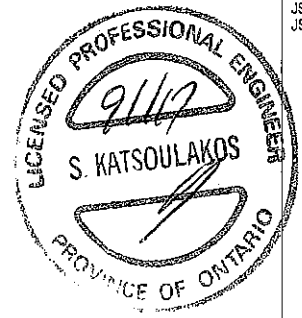
PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1557
	822	2284	1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

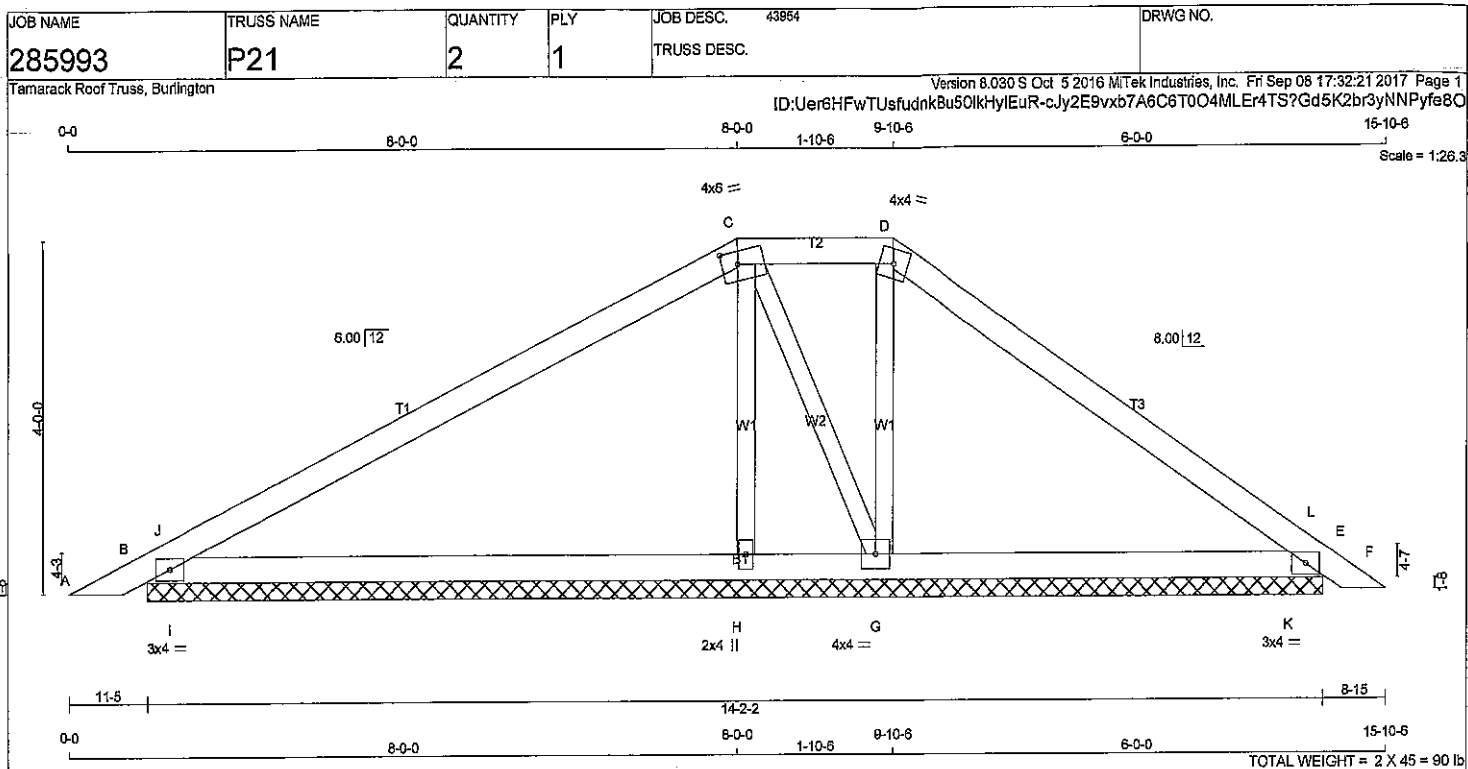
JSI GRIP= 0.26 (B) (INPUT = 0.90)

JSI METAL= 0.07 (H) (INPUT = 1.00)



DWG NO. TAM 45671-17

STRUCTURAL COMPONENT ONLY



LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0	
C	TTWW-m	MT20	4.0	6.0	1.75 2.25
D	TTW-m	MT20	4.0	4.0	
E	TMB1-I	MT20	3.0	4.0	
G	BMW1-I	MT20	4.0	4.0	
H	BMW1+u	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	HORZ	DOWN	HORZ	BRG	IN-SX	IN-SX	IN-SX
B	547	0	547	0	0	14-2-2	14-2-2	14-2-2	14-2-2
E	449	0	449	0	0	14-2-2	14-2-2	14-2-2	14-2-2
H	413	0	413	0	0	14-2-2	14-2-2	14-2-2	14-2-2
G	312	0	312	0	0	14-2-2	14-2-2	14-2-2	14-2-2

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	
B	429	271 / 0	76 / 0	0 / 0	0 / 0	82 / 0	0 / 0	
E	355	219 / 0	66 / 0	0 / 0	0 / 0	70 / 0	0 / 0	
H	372	153 / 0	121 / 0	0 / 0	0 / 0	99 / 0	0 / 0	
G	238	161 / 0	35 / 0	0 / 0	0 / 0	42 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 15	-84.3 -84.3	0.05 (1)	10.00	H-C	-131 / 4	0.03 (1)
B-J	-196 / 0	-84.3 -84.3	0.21 (1)	6.25	C-G	-107 / 0	0.03 (1)
J-C	-253 / 0	-84.3 -84.3	0.55 (1)	6.25	G-D	-157 / 11	0.04 (1)
C-D	-196 / 0	-84.3 -84.3	0.05 (1)	6.25	I-J	-872 / 205	0.00 (1)
D-L	-215 / 0	-84.3 -84.3	0.30 (1)	6.25	K-L	-480 / 170	0.00 (1)
L-E	-199 / 0	-84.3 -84.3	0.11 (1)	6.25			
E-F	0 / 14	-84.3 -84.3	0.03 (1)	10.00			
S-I	0 / 217	-28.0 -28.0	0.42 (1)	10.00			
I-H	0 / 217	-28.0 -28.0	0.42 (1)	10.00			
H-G	0 / 213	-28.0 -28.0	0.37 (1)	10.00			
G-K	0 / 171	-28.0 -28.0	0.25 (1)	10.00			
K-E	0 / 171	-28.0 -28.0	0.24 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.55 (C-J:1), BC=0.42 (H-I:1), WB=0.04 (D-G:1), SSI=0.55 (B-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1867 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.43 (B) (INPUT = 0.90)
JSI METAL= 0.12 (B) (INPUT = 1.00)



DRWG NO. TAN 45672-17
STRUCTURAL
COMPONENT ONLY

LUL/LUS/LJS/HUS/HHUS/HGUS

SIMPSON
Strong-Tie

Standard and Double-Shear Joist Hangers



This product is preferable to similar connectors because of
a) easier installation, b) higher capacities, c) lower installed
cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 258–259.

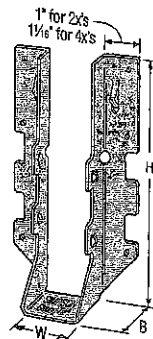
Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 20–24.

Installation:

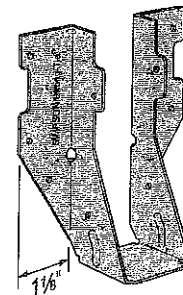
- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1 1/2" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

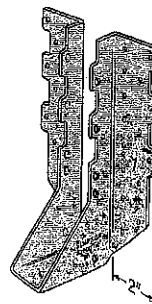
- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on p. 126.



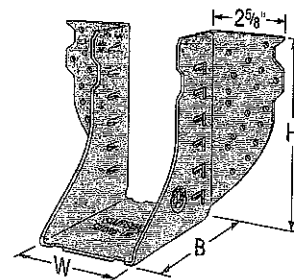
LUS28



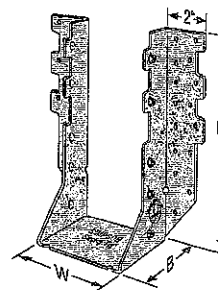
LU26L



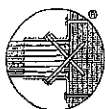
HUS210
(HUS26, HUS28,
and HHUS similar)



HGUS28-2



HHUS210-2



Double-Shear
Nailing
Top View

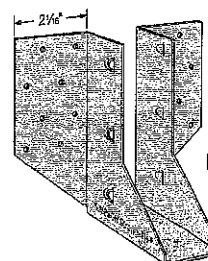
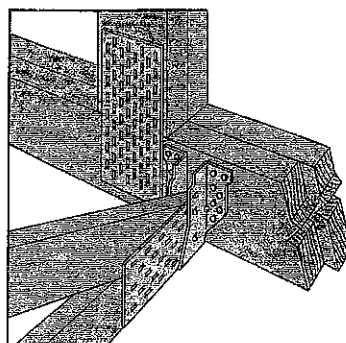


Double-Shear
Nailing
Side View;
Do not
bend tab



Dome Double-Shear
Nailing Side View
(available on
some models)
U.S. Patent 5,603,580

**Typical HUS26
Installation
with Reduced
Heel Height**
(Truss Designer
to provide
fastener quantity
for connecting
multiple members
together)



LJS26DS

Plated Truss Connectors

LUL/LUS/LJS/HUS/HHUS/HGUS**HHUS/HGUS**

See Hanger Options information on pp. 125–127.

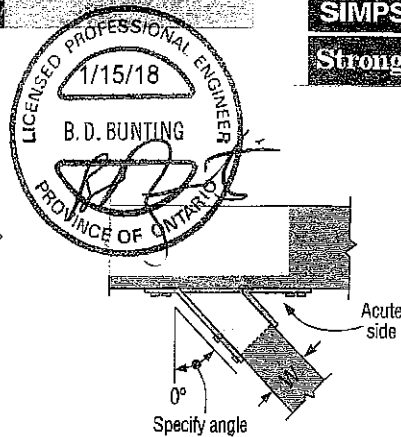
HHUS — Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.62 of the table value
- The joist must be bevel-cut to allow for double-shear nailing

HGUS — Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Joist	Down Resistance	Uplift
W < 2"	Bevel or square cut	0.62 of table value	0.46 of table value
2" < W < 6"	Bevel cut	0.67 of table value	0.41 of table value
2" < W < 6"	Square cut	0.46 of table value	0.41 of table value
W > 6"	Bevel cut	0.75 of table value	0.41 of table value



**Top View HHUS Hanger
Skewed Right**
(joist must be bevel cut)
All joist nails installed on the
outside angle (non-acute side).

Standard and Double-Shear Joist Hangers (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance			
		W	H	B	d _e ³	Header	Joist	D-Fir-L		S-P-F	
								Uplift (K _Q = 1.15) lb.	Normal (K _Q = 1.00) lb.	Uplift (K _Q = 1.15) lb.	Normal (K _Q = 1.00) lb.
								kN	kN	kN	kN
Single 2x Sizes											
LUS24	18	1 1/8	3 1/4	1 1/4	2 1/4	(4) 10d	(2) 10d	710	1625	645	1155
								3.18	7.23	2.87	5.14
LU24L	22	1 1/8	3	1 1/4	2 1/4	(4) 10d	(2) 10d x 1 1/2"	360	1020	320	725
								1.60	4.54	1.42	3.22
LU26L	22	1 1/8	5	1 1/4	4 1/4	(6) 10d	(4) 10d x 1 1/2"	720	1605	645	1140
								3.20	7.14	2.87	5.07
LUS26	18	1 1/8	4 1/4	1 1/4	3 3/4	(4) 10d	(4) 10d	1420	2170	1290	1630
								6.32	9.65	5.74	7.25
HUS26	16	1 1/8	5 3/8	3	3 1/4	(14) 16d	(6) 16d	2705	4940	2065	3875
								11.30	21.97	9.20	17.24
LJS26DS	18	1 1/8	5	3 1/2	4 1/4	(18) 16d	(6) 16d	2055	4265	1460	4115
								9.14	18.97	6.49	18.31
HGUS26	12	1 1/8	5 3/8	5	4 1/4	(20) 16d	(8) 16d	2685	6625	2685	5700
								11.96	29.51	11.96	25.35
LU26L	20	1 1/8	6 1/4	1 1/4	5 1/4	(8) 10d	(6) 10d x 1 1/2"	1140	2185	1020	1550
								5.07	9.72	4.54	6.89
LUS28	18	1 1/8	6 1/4	1 1/4	3 3/4	(6) 10d	(4) 10d	1420	2520	1290	1790
								6.32	11.21	5.74	7.96
HUS28	16	1 1/8	7 1/8	3	6 1/4	(22) 16d	(8) 16d	3605	5365	2675	4345
								16.04	23.86	11.90	19.33
HGUS28	12	1 1/8	7 1/8	5	6 1/4	(36) 16d	(12) 16d	3310	7675	3310	8900
								14.74	34.19	14.74	30.73
LU210L	20	1 1/8	8	1 1/4	7 1/4	(10) 10d	(6) 10d x 1 1/2"	1140	2495	1020	1770
								5.07	11.10	4.54	7.87
LUS210	18	1 1/8	7 1/4	1 1/4	3 3/4	(8) 10d	(4) 10d	1420	2785	1290	2210
								6.32	12.39	5.74	9.83

1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.

2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.

3. d_e is the distance from the bearing seat to the top joist nail.

4. Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-C-N10TRSSCN and/or see installation notes.

5. Nails: 16d = 0.162" dia. x 3 1/2" long. See pp. 27–28 for other nail sizes and information.

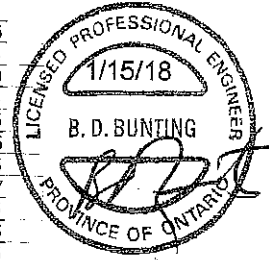
Face-Mount Hangers

SIMPSON
Strong-Tie

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

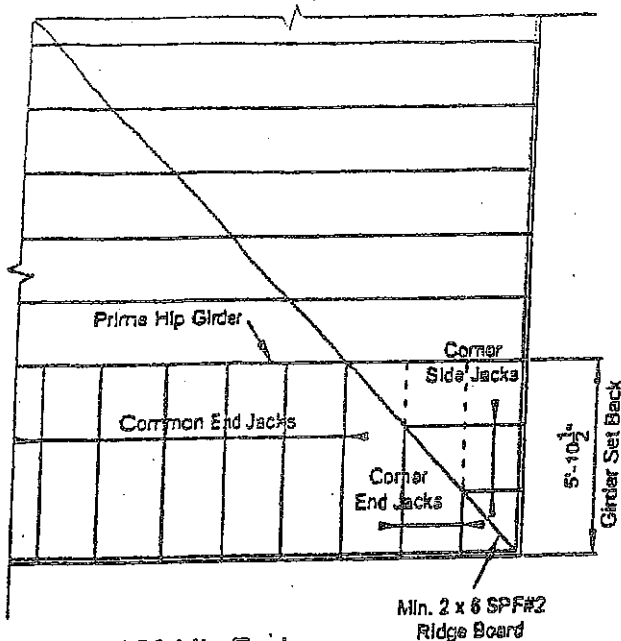
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistances			
		W	H	B	d _o ³	Header	Joist	D-Fir-L		S-P-F	
								Uplift (K _D = 1.15)	Normal (K _D = 1.00)	Uplift (K _D = 1.15)	Normal (K _D = 1.00)
								lb.	lb.	lb.	lb.
								kN	kN	kN	kN
Double 2x Sizes											
LUS24-2	18	3 1/8	3 1/8	2	1 1/2	(4) 16d	(2) 16d	835	2020	590	1435
								3.71	8.99	2.62	6.38
LUS26-2	18	3 1/8	4 1/8	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
								7.65	11.54	6.87	8.54
HHUS26-2	14	3 1/8	5 1/8	3	3 1/8	(14) 16d	(6) 16d	2850	7335	2065	5205
								12.68	32.63	9.20	23.15
HGUS26-2	12	3 1/8	5 7/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
								7.65	14.79	6.87	11.45
HHUS28-2	14	3 1/8	7 1/8	3	6 1/8	(22) 16d	(8) 16d	3765	8940	2675	6345
								16.75	39.77	11.90	28.22
HGUS28-2	12	3 1/8	7 3/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
								11.48	20.02	10.32	14.21
HHUS210-2	14	3 1/8	9 1/8	3	8	(30) 16d	(10) 16d	4670	9660	4235	7000
								20.77	42.97	18.84	31.14
HGUS210-2	12	3 1/8	9 3/8	4	8 1/8	(46) 16d	(16) 16d	6840	14015	4855	10270
								30.43	62.34	21.60	45.69
Triple 2x Sizes											
HGUS26-3	12	4 1/8	5 1/2	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
HGUS28-3	12	4 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
HHUS210-3	14	4 1/8	9	3	7 1/8	(30) 16d	(10) 16d	4670	9670	4235	6865
								20.77	43.02	18.84	30.54
HGUS210-3	12	4 1/8	9 1/4	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400
								30.43	65.14	21.60	46.26
Quadruple 2x Sizes											
HGUS26-4	12	6 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
HGUS28-4	12	6 1/8	7 3/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
HHUS210-4	14	6 1/8	8 1/8	3	7 1/8	(30) 16d	(10) 16d	4670	10155	4235	7210
								20.77	45.17	18.84	32.07
HGUS210-4	12	6 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400
								30.43	65.14	21.60	46.26
HGUS212-4	12	6 1/8	10 1/8	4	10 1/8	(56) 16d	(20) 16d	7640	14995	5425	10645
								33.98	66.70	24.13	47.35
HGUS214-4	12	6 1/8	12 1/8	4	11 1/8	(66) 16d	(22) 16d	10130	16400	7195	11645
								45.06	72.95	32.00	51.80
4x Sizes											
LUS46	18	3 1/8	4 1/8	2	3 1/8	(4) 16d	(4) 16d	1720	2595	1545	1920
								7.65	11.54	6.87	8.54
HHUS46	14	3 1/8	5 1/8	3	3 1/8	(14) 16d	(6) 16d	2540	7335	2065	5205
								11.30	32.63	9.20	23.15
HGUS46	12	3 1/8	5 1/4	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
LUS48	18	3 1/8	6 1/8	2	3 1/8	(6) 16d	(4) 16d	1720	3325	1545	2575
								7.65	14.79	6.87	11.45
HHUS48	14	3 1/8	7 1/8	3	6 1/8	(22) 16d	(8) 16d	3765	8940	2675	6345
								16.75	39.77	11.90	28.22
HGUS48	12	3 1/8	7 3/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
LUS410	18	3 1/8	8 1/8	2	5 1/8	(8) 16d	(6) 16d	2580	4500	2320	3195
								11.48	20.02	10.32	14.21
HGUS410	12	3 1/8	9	4	8 1/8	(46) 16d	(16) 16d	6840	14015	4855	10270
								30.43	62.34	21.60	45.69
HGUS412	12	3 1/8	10 1/8	4	10 1/8	(56) 16d	(20) 16d	7640	14995	5425	10645
								33.98	66.70	24.13	47.35
HGUS414	12	3 1/8	12 1/8	4	11 1/8	(66) 16d	(22) 16d	10130	16400	7195	11645
								45.06	72.95	32.00	51.80



Plated Truss Connectors

MICRO CITY ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242



45° Hip End

LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2

BOTTOM CHORD : 2 x 4 SPF#2

WEBS : 2 x 3 SPF#2

UNLESS OTHERWISE SHOWN

DESIGN LOAD:

TOP CHORD LIVE LOAD : 34.8 P.S.F.

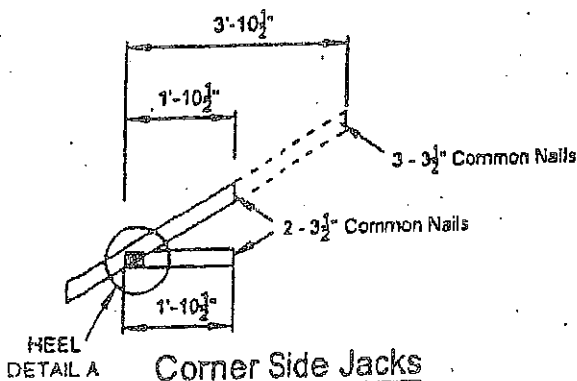
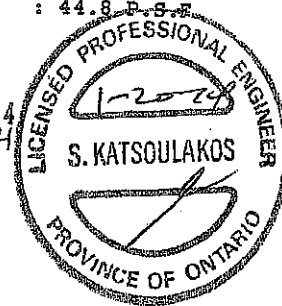
TOP CHORD DEAD LOAD : 3.0 P.S.F.

BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.

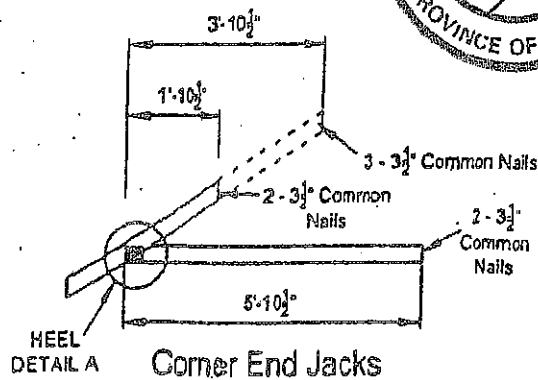
BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD : 44.8 P.S.F.

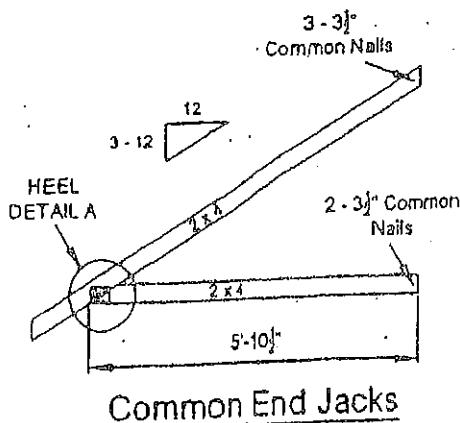
DWG NO TAM 3495.14
STRUCTURAL
COMPONENT ONLY



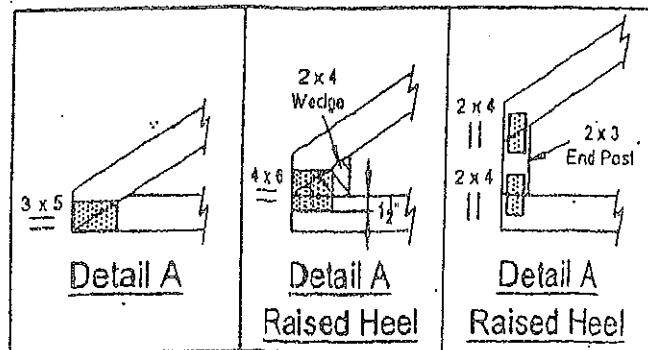
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)

(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

MICRO CITY ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

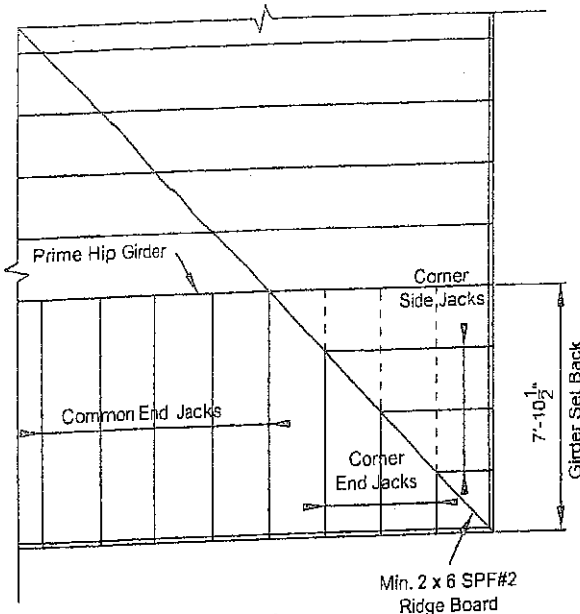
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

DESIGN LOAD:

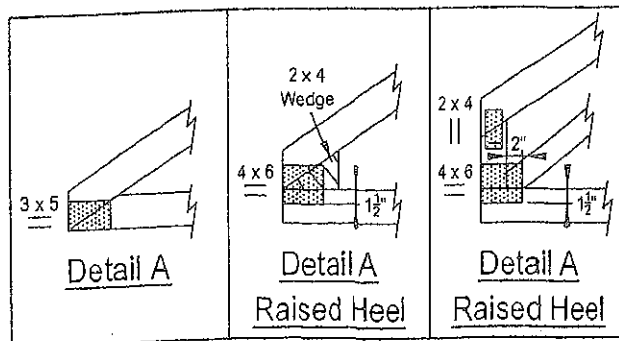
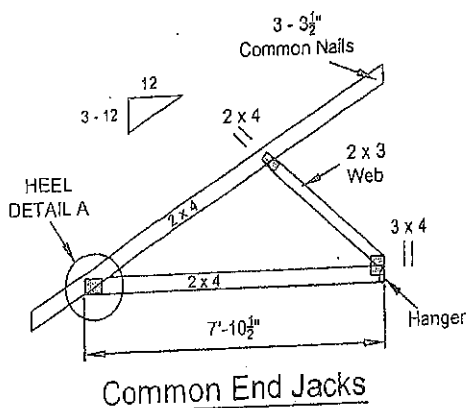
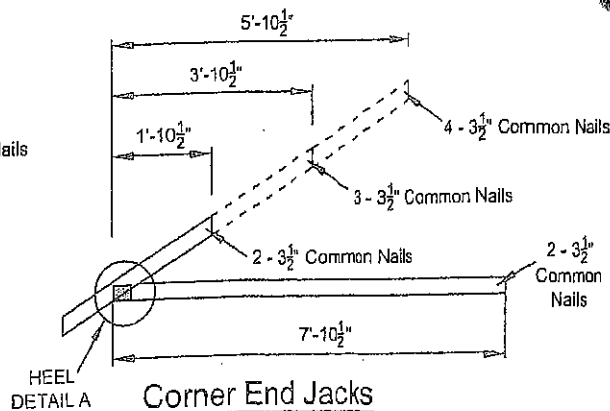
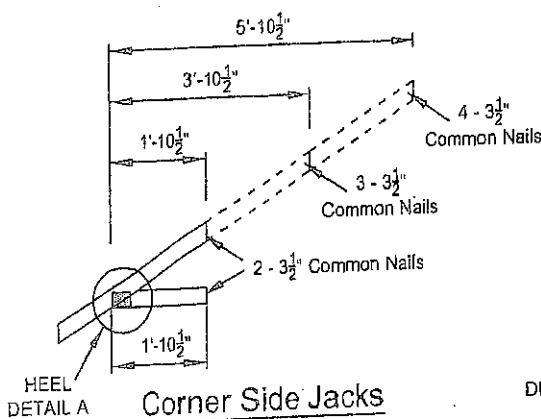
TOP CHORD LIVE LOAD : 40.5 P.S.F.
TOP CHORD DEAD LOAD : 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD : 50.5 P.S.F.



45° Hip End

DWG NO TAM 3504.1
STRUCTURAL
COMPONENT ONLY



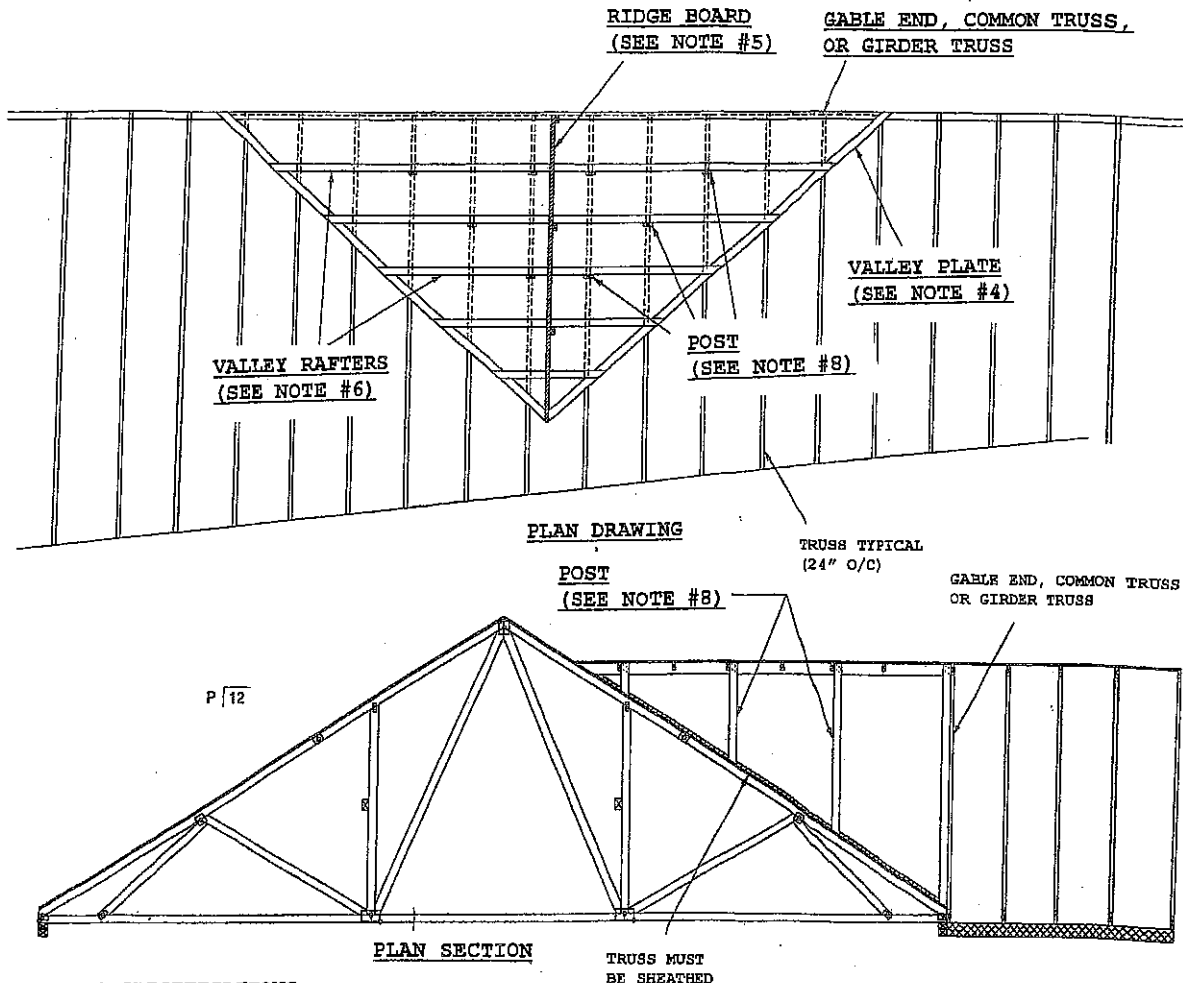
NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

MICRO CITY ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

CONVENTIONAL VALLEY FRAMING DETAIL



GENERAL SPECIFICATIONS:

- (1) WITH THE BASE TRUSSES ERECTED (INSTALLED), APPLY SHEATHING TOP CHORD OF SUPPORTING (BASE) TRUSSES.
- (2) BRACE BOTTOM CHORD AND WEB MEMBERS AS PER PRE-ENGINEERED TRUSS DESIGNS.
- (3) DEFINE VALLEY RIDGE BY RUNNING A LEVEL STRING FROM THE INTERSECTING RIDGE OF THE (a) GABLE END, (b) GIRDER TRUSS OR (c) COMMON TRUSS TO THE ROOF SHEATHING.
- (4) INSTALL 2 X 6 VALLEY PLATES ON FLAT. FASTEN TO EACH SUPPORTING TRUSS WITH (2) 16d (3.5" X 0.131") NAILS.
- (5) SET A 2 X 6 #2 RIDGE BOARD (MAX. 10'-0" RIDGE) OR 2 X 8 #2 SPF RIDGE BOARD (MAX. 20'-0" RIDGE). SUPPORT RIDGE BOARD WITH 2 X 4 POSTS SPACED 48" O/C. BEVEL BOTTOM OF POST TO SET EVENLY ON THE SHEATHING. FASTEN POST TO RIDGE WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST TO ROOF SHEATHING WITH (3) 10d (3" X 0.131") TOE-NAILS.
- (6) FRAME VALLEY RAFTERS FROM VALLEY PLATE TO RIDGE BOARD. MAXIMUM RAFTER SPACING IS 24" O/C. FASTEN VALLEY RAFTER TO RIDGE BEAM WITH (3) 16d (3.5" X 0.131") TOE-NAILS. FASTEN VALLEY RAFTER TO VALLEY PLATE WITH (3) 16d (3.5" X 0.131") TOE-NAILS.
- (7) SUPPORT THE VALLEY RAFTERS WITH 2 X 4 POSTS AT 48" O/C (OR LESS) ALONG EACH RAFTER. INSTALL POSTS IN A STAGGERED PATTERN AS SHOWN ON PLAN DRAWING. ALIGN POSTS WITH TRUSSES BELOW. FASTEN VALLEY RAFTER TO POST WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST THROUGH SHEATHING TO SUPPORTING TRUSSES WITH (2) 16d (3.5" X 0.131") NAILS.
- (8) POSTS SHALL BE 2 X 4 #2 SPF OR BETTER. POSTS EXCEEDING 75" IN HEIGHT SHALL BE INCREASED TO 4 X 4 #2 SPF, OR BETTER, OR BE PRE-ASSEMBLED TWO (2) PLY 2 X 4 #2 SPF OR BETTER FASTENED TOGETHER WITH 2 ROWS OF 10d (3" X 0.131") NAILS AT 6" O/C.
- (9) MAINTAIN A MINIMUM 3/4" LUMBER EDGE DISTANCE WHEN NAILING. NAIL SPACING SHOULD APPROXIMATE A MINIMUM 1-3/4" O/C OR MORE UNLESS NOTED OTHERWISE. ALL CONSTRUCTION TO CONFORM TO ONTARIO BUILDING CODE (CURRENT ADDITION) AT ALL TIMES.

NOTES:

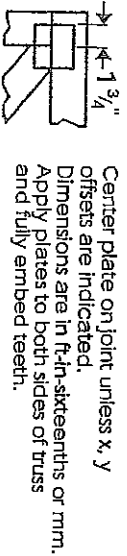
- (10) 48" O/C (MAXIMUM POST SPACING).
- (11) ROOF LIVE LOAD = 34.8 PSF (MAX.)
- (12) ROOF DEAD LOAD = 10.0 PSF (MAX.)
- (13) PART 9 APPLICATION ONLY (ONTARIO BUILDING CODE)
- (14) PART 4 APPLICATION ONLY (ONTARIO BUILDING CODE) WITH APPROVED REVIEW BY LICENSED PROFESSIONAL ENGINEER.
- (15) BASE TRUSS SPACING (24" O/C MAX.)
- (16) ALL PRE-ENGINEERED BASE TRUSS COMPONENTS TO BE SEALED BY LICENSED PROFESSIONAL ENGINEER AND THIS DETAIL TO BE VERIFIED AND APPROVED BY SAME WHEN RIDGE BOARD LENGTH EXCEEDS 12'-0".
- (17) ALL BASE TRUSSES: P = 4 (4/12) - MINIMUM.
- (18) ALL VALLEY RAFTERS: P = 4 (4/12) - MINIMUM.



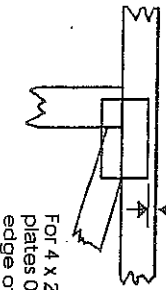
DWC NO T&M 6305.14
STRUCTURAL
COMPONENT ONLY

Symbols

PLATE LOCATION AND ORIENTATION



0-1/16"



This symbol indicates the required direction of slots in connector plates.

* Plate location details available in Mitek software or upon request.

PLATE SIZE

4 X 4

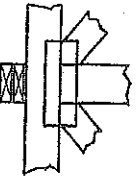
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use 1, I or Eliminator bracing if indicated.

BEARING

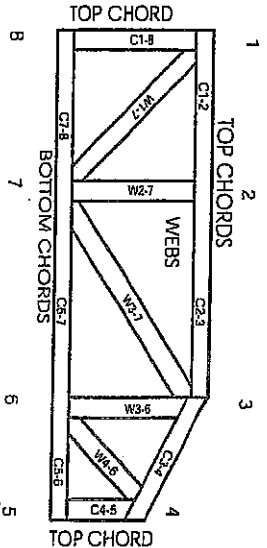


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:
TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

T11996-L, T0319-L, T13270-L, T2691-R

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General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative I, L or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 15% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

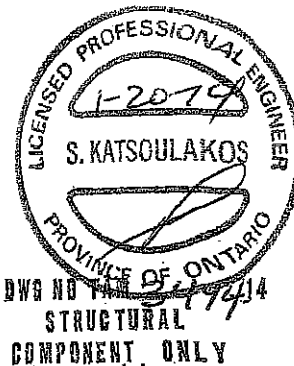
Micro City Engineering Services Inc.
(BCIN: 26064; FIRM BCIN: 29991)

RR #1, Po Box 61

Glencoe, Ontario

NOL 1M0

(519) 287 - 2242; Fax: (519) 287 - 5750 (Call)



Responsibilities:

Micro City Engineering Services is responsible for the design of trusses as individual components.

It is the responsibilities of others to ascertain that the design loads utilized on this (these) drawing(s) meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdiction over such decisions.

All dimensions are to be verified by the owner, contractor, architect, or other authority having input over such decisions prior to truss component manufacture. At no time shall Micro City Engineering Services Inc. or its employees be responsible for dimension errors.

Micro City Engineering Services Inc. bears no responsibility for the erection of any truss components. Persons erecting truss components are cautioned to seek professional advice regarding temporary and permanent bracing systems and to be **totally** familiar with all aspects of truss erection prior to proceeding on **any** truss component erection job. Any bracing shown on Micro City Engineering Services Inc. or Tamarack Roof Trusses Inc. sealed or unsealed truss component drawings is specified for the single truss component in question and is identified as an integral part of the design for that particular truss component but is **not** meant to represent the only required bracing for that particular truss component when installed as a component in a series of truss components in a roof truss system.

It is the truss manufacturer's responsibility to ensure that trusses are manufactured in accordance with Micro City Engineering Services Inc. specifications outlined below:

SPECIFICATIONS:

Truss components sealed by Micro City Engineering Services Inc. must conform to the relevant sections of the current Building Code of Ontario and Canada (Part 4 or Part 9) or the current Farm Building Code of Canada in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the Truss Plate Institute of Canada (TPIC). All unit lumber and nailing stresses identified on truss component design drawings and/or used in the design of individual truss components shall conform to the current CSA Wood Design standard identified in the current Building Code and TPIC Design Standards.

The lumber used to manufacture any truss component is to conform to the specified size and grade identified on the truss drawing.

The lumber used in the manufacture of any truss component is not to exceed 19% during its service use unless specifically noted on the truss drawing.

The lumber used in the manufacture of any truss component is not to be treated with any chemicals during its service life unless specifically noted on the truss drawing.

Connector plates shall be applied to both faces of the truss component at each joint and shall be positioned exactly as specified.

The top chord of any truss component is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the sealed truss component drawing but not exceeding 24" o/c (Part 9 design) and not exceeding 48" o/c (Part 4 or Agricultural design).

When a truss component is to be installed with no rigid ceiling attached directly to the bottom chord, then the bottom chord is to be laterally braced at intervals not exceeding 3m (or 10'-0").

All sealed or unsealed truss component drawings provided by Micro City Engineering Services Inc. Or Tamarack Roof Trusses Inc. should be read in conjunction with the following:

Warning-Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473C rev 10-'08 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer - not the truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component Safety Information available from the Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA, 22314.