ASPHALT SHINGLES
12"FINISH O.H.
R.T.M.C
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:

HGUS26-2 -(XX)
LJS26DS -(V)

6/12 PITCHES (TYP.)
UNLESS NOTED

DENOTES:

* -1'-6"-0 HIGHER PLATE

ALL B- 2-2X10 BEAMS (FLUSH)

HIGHGROVE-5E
EL:1 - UNIT1

285349

HIGHGROVE-4
EL:1- UNIT2

285350

HIGHGROVE-3
EL:1 (REV)- UNIT3

285351

HIGHGROVE-3
EL:1 - UNIT4

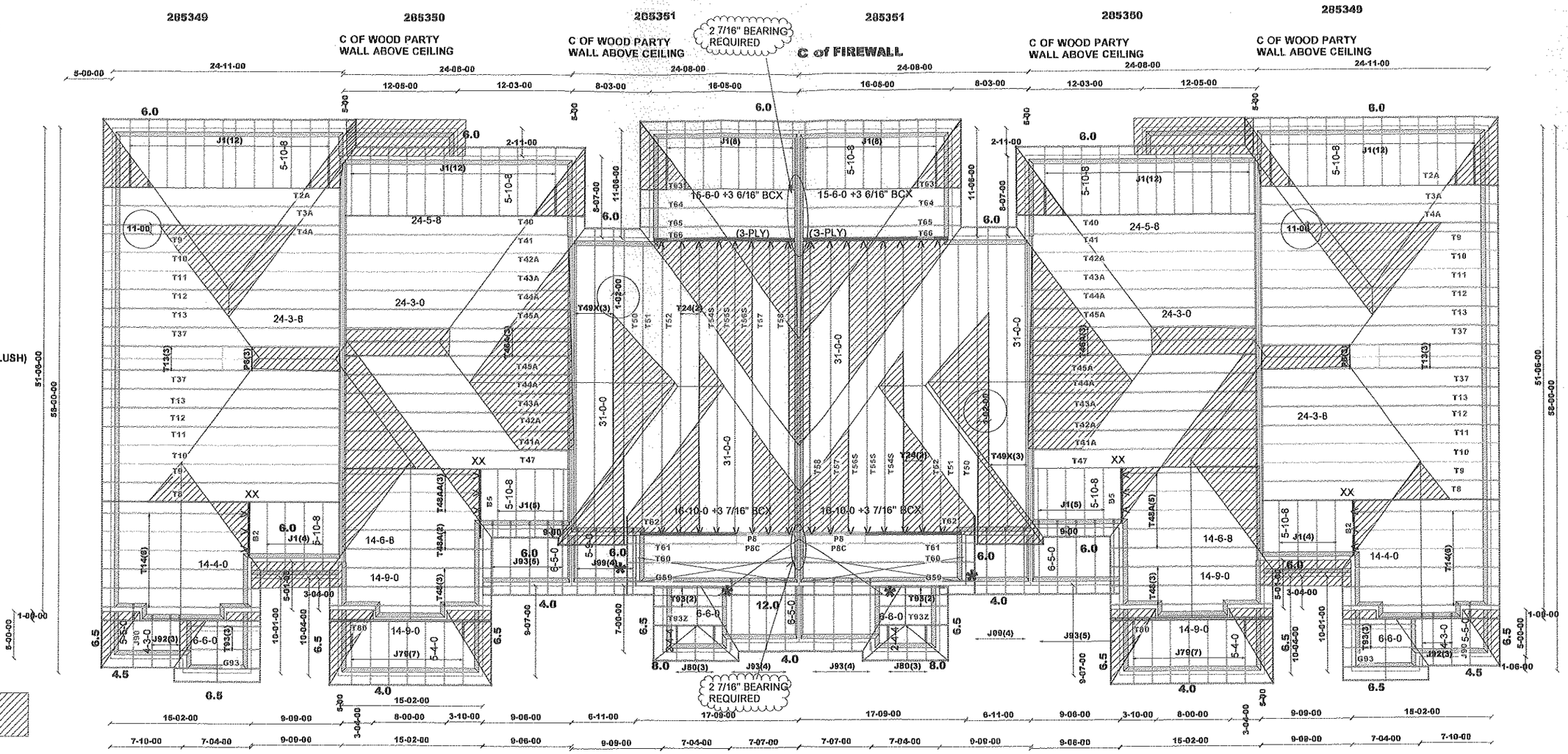
285351

HIGHGROVE-4
EL:1(REV) - UNIT5

285350

HIGHGROVE-5E
EL:1(REV) - UNIT6

285349



T-170534

DENOTES:
CONVENTIONAL FRAMING

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION
ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"o.c.
WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.
POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE
BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN LOADS:

SNOW LOAD 25.6 PSF
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

TAMARACK

Job Track: 43954	Builder / Location: GREENPARK HOMES / WATERDOWN	Model / Elevation: BLOCK203/ UNIT1 TO 6 (REVISED)	Mitek ver 7.5.0
Layout ID: 285348	Project: RUSSEL GARDENS PHASE 2	THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTLILIZED FOR ANY OTHER PURPOSE.	
Plan Log: 93446	Date: 05/26/18 Designer: sonny		

TAMARACK LUMBER INC
ALPA LUMBER GROUP

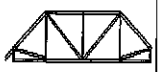
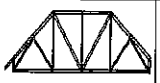
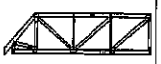
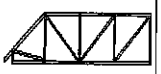
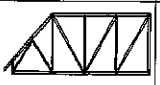
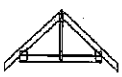
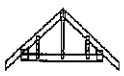
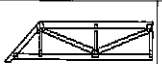
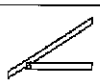
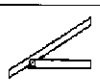
Delivery Shiplist

DATE	04/18/18
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 285349 LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH 2 SUB-BUILDER:
MODEL: HIGHGROVE 5E ELEVATION: EL:1- UNIT1 OR UNIT6

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T2A HIP GIRDER	8.00 0.00	24-03-08	04-01-04	2 X 6	2 X 6	01-03-08 00-00-00	01-04-13 01-06-08	142.19 87.67		
	1	T3A HIP	8.00 0.00	24-03-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	94.81 60.17		
	1	T4A HIP	8.00 0.00	24-03-08	06-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	103.53 66.33		
	1	T8 HALF HIP	8.00 0.00	24-03-08	04-01-04	2 X 6	2 X 6	01-03-08 00-00-00	01-04-13 04-01-04	138.05 86.67		
	2	T9 HALF HIP	8.00 0.00	24-03-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-01-04	197.10 123.66		
	2	T10 HALF HIP	8.00 0.00	24-03-08	06-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 06-01-04	212.66 133.34		
	2	T11 HALF HIP	8.00 0.00	24-03-08	07-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 07-01-04	225.56 142.68		
	2	T12 HALF HIP	8.00 0.00	24-03-08	08-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 08-01-04	234.64 146.34		
	5	T13 HALF HIP	8.00 0.00	24-03-08	09-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 09-01-04	614.40 379.15		
	2	T37 HALF HIP	8.00 0.00	24-03-08	10-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 10-01-04	257.54 159.34		
	6	T14 COMMON	8.00 0.00	14-04-00	06-02-02	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	370.56 234.00		
	3	T93 COMMON	6.50 0.00	06-06-00	02-04-09	2 X 4	2 X 4	01-03-08 01-03-08	00-07-07 00-07-07	66.60 45.00		
	1	G93 COMMON	6.50 0.00	06-06-00	02-04-09	2 X 4	2 X 4	01-03-08 01-03-08	00-07-07 00-07-07	23.39 16.00		
	3	P6 PIGGYBACK	8.00 0.00	11-11-15	01-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-00-00 02-00-00	120.33 77.01		
	16	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	268.64 170.72		
	1	J90 MONOPITCH	4.50 0.00	05-05-00	02-05-00	2 X 4	2 X 4	01-03-08 00-00-00	00-04-10 02-05-00	14.47 9.33		
	3	J92 JACK	4.50 0.00	04-03-00	01-11-12	2 X 4	2 X 4	01-03-08 00-00-00	00-04-10 01-11-12	35.16 24.00		

TOTAL # TRUSS= **52.00** TOTAL BFT OF ALL TRUSSES= **1961.41** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3119.63** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16

TAMARACK LUMBER INC
ALPA LUMBER GROUP

Delivery Shiplist

DATE	04/18/18
SALES REP	Mario

JOB TRACK: 43954 **LAYOUT ID:** 285349 **LOCATION:**
BUILDER: GREENPARK - RUSSELL GARDENS PH 2 **SUB-BUILDER:**
MODEL: HIGHGROVE 5E **ELEVATION:** EL:1- UNIT1 OR UNIT6

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	HGUS26-2	
3	Hangers	LJS26DS	

TOTAL # ITEMS= **4.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

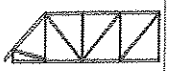
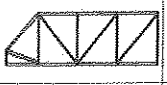
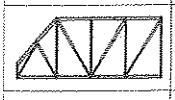
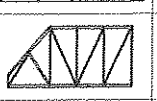
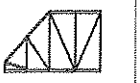
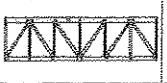
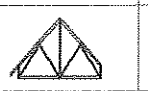
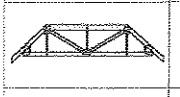
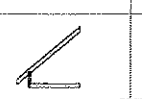
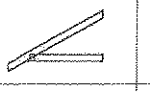
Delivery Shiplist

DATE	06/26/18
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 285350 LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH SUB-BUILDER:
MODEL: HIGHGROVE 4 ELEVATION: EL:1- UNIT2 OR UNIT5

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T40 HALF HIP	8.00 / 0.00	24-05-08	04-01-04	2 X 6	2 X 6	01-03-08 / 00-00-00	01-04-13 / 04-01-04	136.90 / 84.33		
	1	T41 HALF HIP	8.00 / 0.00	24-05-08	05-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 05-01-04	99.05 / 61.83		
	1	T41A HALF HIP	8.00 / 0.00	24-03-00	05-01-04	2 X 4	2 X 4	00-00-00 / 00-00-00	01-06-08 / 05-01-04	96.77 / 61.17		
	2	T42A HALF HIP	8.00 / 0.00	24-03-00	06-01-04	2 X 4	2 X 4	00-00-00 / 00-00-00	01-06-08 / 06-01-04	209.24 / 132.00		
	2	T43A HALF HIP	8.00 / 0.00	24-03-00	07-01-04	2 X 4	2 X 4	00-00-00 / 00-00-00	01-06-08 / 07-01-04	222.10 / 141.34		
	2	T44A HALF HIP	8.00 / 0.00	24-03-00	08-01-04	2 X 4	2 X 4	00-00-00 / 00-00-00	01-06-08 / 08-01-04	231.32 / 146.34		
	2	T45A HALF HIP	8.00 / 0.00	24-03-00	09-01-04	2 X 4	2 X 4	00-00-00 / 00-00-00	01-06-08 / 09-01-04	242.42 / 150.34		
	3	T46A HALF HIP	8.00 / 0.00	24-03-00	10-01-04	2 X 4	2 X 4	00-00-00 / 00-00-00	01-06-08 / 10-01-04	381.24 / 237.00		
	1	T47 FLAT GIRDER	0.00 / 0.00	24-03-00	04-01-04	2 X 6	2 X 6	00-00-00 / 00-00-00	04-01-04 / 04-01-04	145.78 / 90.67		
	3	T48 COMMON	8.00 / 0.00	14-09-00	06-03-13	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	189.99 / 117.00		
	2	T48A COMMON	8.00 / 0.00	14-06-08	06-03-13	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 01-06-08	122.14 / 76.66		
	3	T48AA COMMON	8.00 / 0.00	14-06-08	06-03-13	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 01-06-08	183.21 / 114.99		
	1	T80 HIP GIRDER	6.50 / 0.00	14-09-00	02-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	00-07-07 / 00-07-07	52.40 / 33.33		
	17	J1 JACK-OPEN	6.00 / 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 04-01-04	285.43 / 181.39		
	7	J79 JACK-OPEN	4.00 / 0.00	05-04-00	02-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	00-03-15 / 02-01-04	99.05 / 65.31		
	5	J93 MONOPITCH	4.00 / 0.00	06-05-00	02-05-10	2 X 4	2 X 4	01-03-08 / 00-00-00	00-03-15 / 02-05-10	90.80 / 58.35		

TOTAL # TRUSS= **53** TOTAL BFT OF ALL TRUSSES= **1752.05** BFT. TOTAL WEIGHT OF ALL TRUSSES= **2787.84** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16

Delivery Shiplist

DATE	06/26/18
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 285350 LOCATION:

BUILDER: GREENPARK - RUSSELL GARDENS PH SUB-BUILDER:

MODEL: HIGHGROVE 4 ELEVATION: EL:1- UNIT2 OR UNIT5

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	HGUS26-2	
3	Hangers	LJS26DS	

TOTAL # ITEMS= 4

TAMARACK LUMBER INC
ALPA LUMBER GROUP

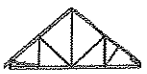
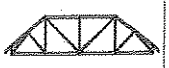
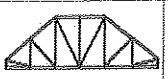
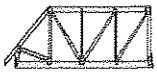
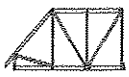
Delivery Shiplist

DATE	06/26/18
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 285351 LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH SUB-BUILDER:
MODEL: HIGHGROVE 3 ELEVATION: EL:1 UNIT3 OR UNIT4

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	2	T24 COMMON	6.00 / 0.00	31-00-00	08-11-00	2 X 4	2 X 4	01-03-08 / 01-03-08	01-02-00 / 01-02-00	243.10 / 153.00		
	3	T49X COMMON	6.00 / 0.00	31-00-00	08-08-12	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 00-09-08	358.56 / 222.00		
	1	T50 HIP	6.00 / 0.00	31-00-00	05-01-02	2 X 4	2 X 4	01-03-08 / 01-03-08	01-02-00 / 01-02-00	118.98 / 75.17		
	1	T51 HIP	6.00 / 0.00	31-00-00	06-05-02	2 X 4	2 X 4	00-00-00 / 00-00-00	01-02-00 / 01-02-00	126.20 / 79.00		
	1	T52 HIP	6.00 / 0.00	31-00-00	07-09-02	2 X 4	2 X 4	00-00-00 / 00-00-00	01-02-00 / 01-02-00	124.59 / 78.17		
	1	T54S ROOF	6.00 / 0.00	31-00-00	08-04-08	2 X 4	2 X 4	00-00-00 / 00-00-00	05-11-13 / 08-04-08	154.58 / 97.67		
	1	T55S ROOF	6.00 / 0.00	31-00-00	09-08-08	2 X 4	2 X 4	00-00-00 / 00-00-00	06-11-00 / 09-08-08	172.69 / 108.83		
	1	T56S ROOF	6.00 / 0.00	31-00-00	11-00-08	2 X 4	2 X 4	00-00-00 / 00-00-00	06-11-00 / 11-00-08	190.92 / 119.83		
	1	T57 HALF HIP	6.00 / 0.00	31-00-00	09-11-13	2 X 4	2 X 4	00-00-00 / 00-00-00	06-11-00 / 09-11-13	178.19 / 111.84		
	1	T58 HALF HIP	6.00 / 0.00	31-00-00	11-03-13	2 X 4	2 X 4	00-00-00 / 00-00-00	06-11-00 / 11-03-13	193.92 / 116.67		
	1	G59 HALF HIP	8.00 / 0.00	16-10-00	08-04-08	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 08-04-08	88.51 / 55.50		
	1	T60 HALF HIP	8.00 / 0.00	16-10-00	10-04-08	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 00-00-04	95.69 / 59.67		
	1	T61 PIGGYBACK	8.00 / 0.00	16-10-00	09-10-08	2 X 4	2 X 4	01-03-08 / 00-00-00	02-10-13 / 00-00-04	92.47 / 58.33		
	1 / 3 Ply	T62 PIGGYBACK	8.00 / 0.00	16-10-00	09-10-08	2 X 4	2 X 6	00-00-00 / 00-00-00	02-10-13 / 00-00-04	352.44 / 218.01		
	1	T63 HALF HIP	8.00 / 0.00	15-06-00	04-01-04	2 X 4	2 X 6	01-03-08 / 00-00-00	01-04-13 / 00-00-00	77.81 / 48.83		
	1	T64 HALF HIP	8.00 / 0.00	15-06-00	05-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 00-00-00	67.71 / 43.00		
	1	T65 HALF HIP	8.00 / 0.00	15-06-00	06-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 00-00-00	70.02 / 44.33		
	1 / 3 Ply	T66 HALF HIP	8.00 / 0.00	15-06-00	06-08-12	2 X 6	2 X 6	00-00-00 / 00-00-00	01-04-13 / 00-00-00	334.65 / 201.00		

TAMARACK LUMBER INC

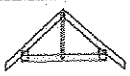
Delivery Shiplist

DATE	06/26/18
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 285351 LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH SUB-BUILDER:
MODEL: HIGHGROVE 3 ELEVATION: EL:1 UNIT3 OR UNIT4

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	2	T93 COMMON	6.50 0.00	06-06-00	02-04-09	2 X 4	2 X 4	01-03-08 01-03-08	00-07-07 00-07-07	44.40 30.00		
	1	T93Z COMMON	6.50 0.00	06-06-00	02-04-09	2 X 4	2 X 4	01-03-08 01-03-08	00-07-07 00-07-07	22.20 15.00		
	1 3 Ply	P8 PIGGYBACK	8.00 0.00	05-07-09	04-01-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 04-01-08	57.36 36.00		
	1	P8C PIGGYBACK	8.00 0.00	05-07-08	03-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 03-09-04	18.96 12.00		
	8	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	134.32 85.36		
	3	J80 JACK-OPEN	8.00 0.00	02-04-04	02-04-08	2 X 4	2 X 4	01-03-08 00-00-00	00-09-10 02-04-08	24.93 18.00		
	4	J93 MONOPITCH	4.00 0.00	06-05-00	02-05-10	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-05-10	72.64 46.68		
	4	J99 MONOPITCH	4.00 0.00	05-09-00	02-02-15	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-02-15	60.52 37.32		

TOTAL # TRUSS= **51.00** TOTAL BFT OF ALL TRUSSES= **2171.21** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3476.36** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
16	Hangers	LJS26DS	

TOTAL # ITEMS= **16.00**

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285349	T2A	1	1	TRUSS DESC.	

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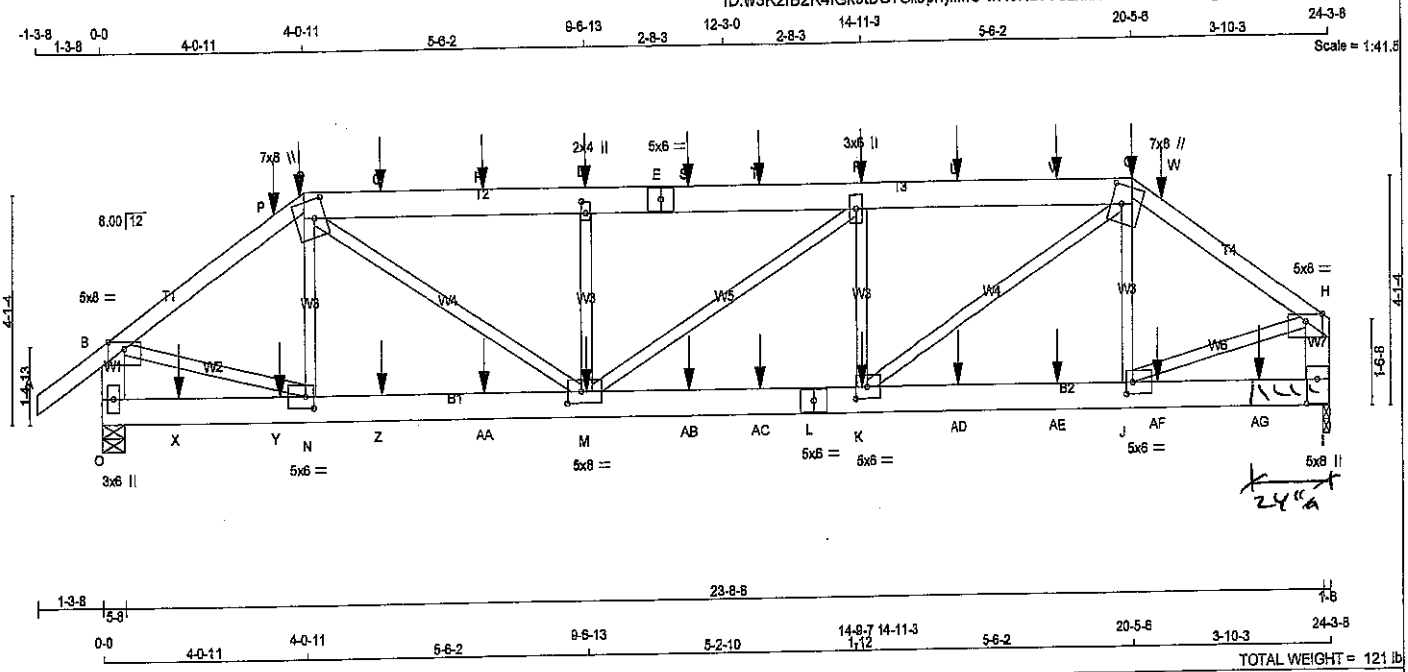
Tamarack Roof Truss, Burlington

ID:w3K2IB2K4fGk9tDO7SkJphylfMC-wH0RdvVdLxhl4LcOJO6nFCZcgG8QBBscl7NHYbznu5B



TOTAL WEIGHT = 121 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - G	2x6	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
O - B	2x6	DRY	No.2	SPF
I - H	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	7.0	8.0	4.00	2.50
D	TMW+w	MT20	2.0	4.0	2.50	1.00
E	TS-t	MT20	5.0	8.0		
F	TMWW+t	MT20	3.0	8.0		
G	TTWW+m	MT20	7.0	8.0	4.00	2.50
H	TMW-p	MT20	5.0	8.0	Edge	
I	BMV1+p	MT20	5.0	8.0	5.25	2.50
J	BMWW-t	MT20	5.0	8.0	2.50	1.50
K	BMWW-t	MT20	5.0	8.0	2.50	2.75
L	BS-t	MT20	5.0	6.0		
M	BMWWW-t	MT20	5.0	8.0	2.50	3.25
N	BMWW-t	MT20	5.0	6.0	2.50	2.00
O	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 122.8 lbs FACTORED DOWN AT 3-6-12, 101.4 lbs FACTORED DOWN AT 5-6-12, 101.4 lbs FACTORED DOWN AT 7-6-12, 101.4 lbs FACTORED DOWN AT 9-6-12, 101.4 lbs FACTORED DOWN AT 11-6-12, 101.4 lbs FACTORED DOWN AT 12-11-4, 101.4 lbs FACTORED DOWN AT 14-11-4, 101.4 lbs FACTORED DOWN AT 16-11-4, 101.4 lbs FACTORED DOWN AT 18-11-4, 122.8 lbs FACTORED DOWN AT 20-11-4, AND 244.5 lbs FACTORED DOWN AT 4-0-11, AND 244.5 lbs FACTORED DOWN AT 20-5-8 ON TOP CHORD, AND 89.9 lbs FACTORED DOWN AT 1-6-12, 89.9 lbs FACTORED DOWN AT 3-6-12, 89.9 lbs FACTORED DOWN AT 5-6-12, 89.9 lbs FACTORED DOWN AT 7-6-12, 89.9 lbs FACTORED DOWN AT 9-6-12, 89.9 lbs FACTORED DOWN AT 11-6-12, 89.9 lbs FACTORED DOWN AT 12-11-4, 89.9 lbs FACTORED DOWN AT 14-11-4, 89.9 lbs FACTORED DOWN AT 16-11-4, 89.9 lbs FACTORED DOWN AT 16-11-4, AND 89.9 lbs FACTORED DOWN AT 20-11-4, AND 89.9 lbs FACTORED DOWN AT 22-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	2483	0	2483	0	0	5-8	5-8
I	2385	0	2385	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	2015	1157 / 0	433 / 0	0 / 0	0 / 0	425 / 0	0 / 0
I	1951	1094 / 0	438 / 0	0 / 0	0 / 0	420 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

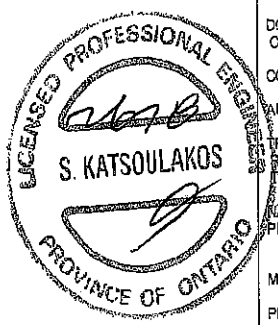
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
FR-TO					
A- B	0 / 32	-84.3	-84.3	0.12 (1)	10.00
B- P	-2793 / 0	-84.3	-84.3	0.47 (1)	3.66
P- C	-2793 / 0	-84.3	-84.3	0.47 (1)	3.66
C- Q	-3935 / 0	-84.3	-84.3	0.38 (1)	3.97
Q- R	-3935 / 0	-84.3	-84.3	0.38 (1)	3.97
R- D	-3935 / 0	-84.3	-84.3	0.38 (1)	3.97
D- E	-3935 / 0	-84.3	-84.3	0.35 (1)	4.01
E- S	-3935 / 0	-84.3	-84.3	0.35 (1)	4.01
S- T	-3935 / 0	-84.3	-84.3	0.35 (1)	4.01
T- F	-3935 / 0	-84.3	-84.3	0.35 (1)	4.01
F- U	-3914 / 0	-84.3	-84.3	0.37 (1)	3.99
U- V	-3914 / 0	-84.3	-84.3	0.37 (1)	3.99
V- G	-3914 / 0	-84.3	-84.3	0.37 (1)	3.99
G- W	-2867 / 0	-84.3	-84.3	0.43 (1)	3.78
W- H	-2687 / 0	-84.3	-84.3	0.43 (1)	3.78
Q- B	-2418 / 0	0.0	0.0	0.17 (1)	6.59
I- H	-2322 / 0	0.0	0.0	0.17 (1)	6.70
O- X	0 / 0	-28.0	-28.0	0.13 (3)	10.00
X- Y	0 / 0	-28.0	-28.0	0.13 (3)	10.00
Y- N	0 / 0	-28.0	-28.0	0.13 (3)	10.00
N- Z	0 / 2344	-28.0	-28.0	0.39 (1)	10.00
Z-AA	0 / 2344	-28.0	-28.0	0.39 (1)	10.00
AA- M	0 / 2344	-28.0	-28.0	0.39 (1)	10.00
M-AB	0 / 3913	-28.0	-28.0	0.62 (1)	10.00
AB-AC	0 / 3913	-28.0	-28.0	0.62 (1)	10.00
AC- L	0 / 3913	-28.0	-28.0	0.62 (1)	10.00
L- K	0 / 3913	-28.0	-28.0	0.62 (1)	10.00
K-AD	0 / 2253	-28.0	-28.0	0.37 (1)	10.00
AD-AE	0 / 2253	-28.0	-28.0	0.37 (1)	10.00
AE- J	0 / 2253	-28.0	-28.0	0.37 (1)	10.00
J-AF	0 / 0	-28.0	-28.0	0.13 (3)	10.00
AF-AG	0 / 0	-28.0	-28.0	0.13 (3)	10.00
AG- I	0 / 0	-28.0	-28.0	0.13 (3)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
N- C	-300 / 167	0.07 (1)
C- M	0 / 1912	0.47 (1)
M- D	-825 / 0	0.20 (1)
K- F	-848 / 0	0.20 (1)
K- G	0 / 1995	0.49 (1)
J- G	-392 / 118	0.09 (1)
B- N	0 / 2419	0.60 (1)
J- H	0 / 2358	0.58 (1)
M- F	0 / 26	0.01 (1)



DWG NO. TAM 8092-16
STRUCTURAL COMPONENT ONLY

[M]

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

*** NON STANDARD GIRDER ***
ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.47/1.00 (B-C:1) , BC=0.62/1.00 (K-M:1) , WB=0.60/1.00 (B-N:1) , SSI=0.25/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285349	T2A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington
ID:w3K2IB2K4fGk9tDO7SkJphylfMC-wH0RdvVdLxhI4LcOJO8nFCZcgG8QBBscl7NHYbznu5B

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	4-0-11	-23	-25	—	FRONT	VERT	DEAD
C	4-0-11	-222	-222	—	FRONT	VERT	SNOW
D	9-6-12	-101	-101	—	BACK	VERT	TOTAL
F	14-11-4	-101	-101	—	BACK	VERT	TOTAL
G	20-5-6	-23	-25	—	FRONT	VERT	DEAD
G	20-5-6	-222	-222	—	FRONT	VERT	SNOW
K	14-11-4	-40	-70	—	BACK	VERT	TOTAL
M	9-8-12	-40	-70	—	BACK	VERT	TOTAL
P	3-6-12	-123	-123	—	BACK	VERT	TOTAL
Q	5-6-12	-101	-101	—	BACK	VERT	TOTAL
R	7-6-12	-101	-101	—	BACK	VERT	TOTAL
S	11-6-12	-101	-101	—	BACK	VERT	TOTAL
T	12-11-4	-101	-101	—	BACK	VERT	TOTAL
U	16-11-4	-101	-101	—	BACK	VERT	TOTAL
V	18-11-4	-101	-101	—	BACK	VERT	TOTAL
W	20-11-4	-123	-123	—	BACK	VERT	TOTAL
X	1-6-12	-40	-70	—	BACK	VERT	TOTAL
Y	3-6-12	-40	-70	—	BACK	VERT	TOTAL
Z	5-6-12	-40	-70	—	BACK	VERT	TOTAL
AA	7-6-12	-40	-70	—	BACK	VERT	TOTAL
AB	11-6-12	-40	-70	—	BACK	VERT	TOTAL
AC	12-11-4	-40	-70	—	BACK	VERT	TOTAL
AD	16-11-4	-40	-70	—	BACK	VERT	TOTAL
AE	18-11-4	-40	-70	—	BACK	VERT	TOTAL
AF	20-11-4	-40	-70	—	BACK	VERT	TOTAL
AG	22-11-4	-40	-70	—	BACK	VERT	TOTAL

JSI GRIP= 0.90 (N) (INPUT = 0.90)
JSI METAL= 0.83 (L) (INPUT = 1.00)

SCAB JOINT # I & H WITH 2 X 6 #2
SPF 24" LONG 1 SIDE(S) USING 2
ROW(S) 2/2 " ARDOX SPIRAL ~~COMMON WIRE~~
NAILS AT 3" O/C STAGGERED (2) NAILS/SCAB).
REQUIRED FOR 1½" BRG PROVISION.

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM8092-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285862	T3A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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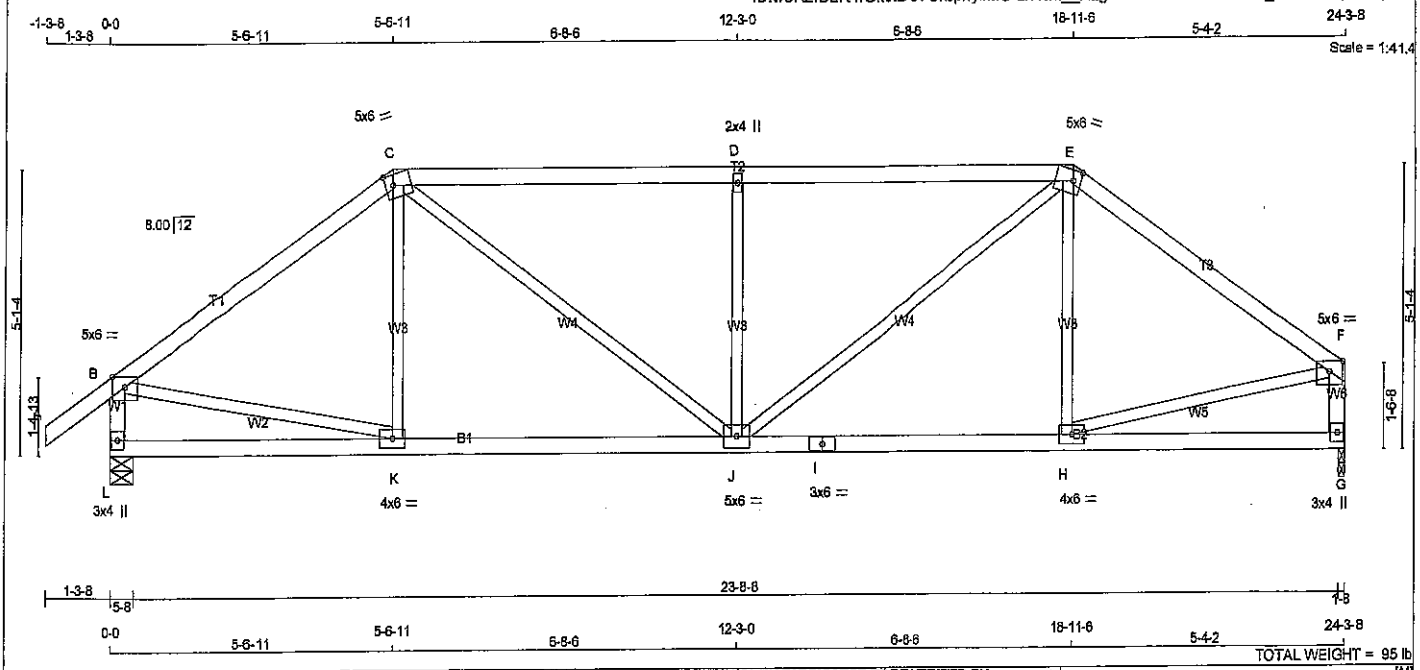

TOTAL WEIGHT = 95 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TTWW-m	MT20	5.0	6.0	2.25	1.75
D	TMW+w	MT20	2.0	4.0		
E	TTWW-m	MT20	5.0	6.0	2.25	1.75
F	TMVW-p	MT20	5.0	8.0	Edge	
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	5.0	8.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1479	0	1479	0	0	5-8	5-8
G	1363	0	1363	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1198	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0
G	1119	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	K- C	-45 / 211	0.05 (3)
B- C	-1495 / 0	-84.3	-84.3	0.55 (1)	4.69	C- J	0 / 728	0.16 (1)
C- D	-1833 / 0	-84.3	-84.3	0.80 (1)	3.88	J- D	-892 / 0	0.27 (1)
D- E	-1833 / 0	-84.3	-84.3	0.80 (1)	3.88	J- E	0 / 768	0.17 (1)
E- F	-1456 / 0	-84.3	-84.3	0.50 (1)	4.81	H- E	-81 / 187	0.04 (3)
L- B	-1416 / 0	0.0	0.0	0.15 (1)	6.85	B- K	0 / 1285	0.28 (1)
G- F	-1305 / 0	0.0	0.0	0.14 (1)	7.08	H- F	0 / 1239	0.28 (1)
L- K	0 / 0	-28.0	-28.0	0.25 (3)	10.00			
K- J	0 / 1241	-28.0	-28.0	0.40 (2)	10.00			
J- I	0 / 1208	-28.0	-28.0	0.39 (2)	10.00			
I- H	0 / 1208	-28.0	-28.0	0.39 (2)	10.00			
H- G	0 / 0	-28.0	-28.0	0.24 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.80/1.00 (D-E:1) , BC=0.40/1.00 (J-K:2) , WB=0.28/1.00 (B-K:1) , SSI=0.27/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (E) (INPUT = 0.90)
JSI METAL= 0.36 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
1/16/18
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 4008-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285862	T4A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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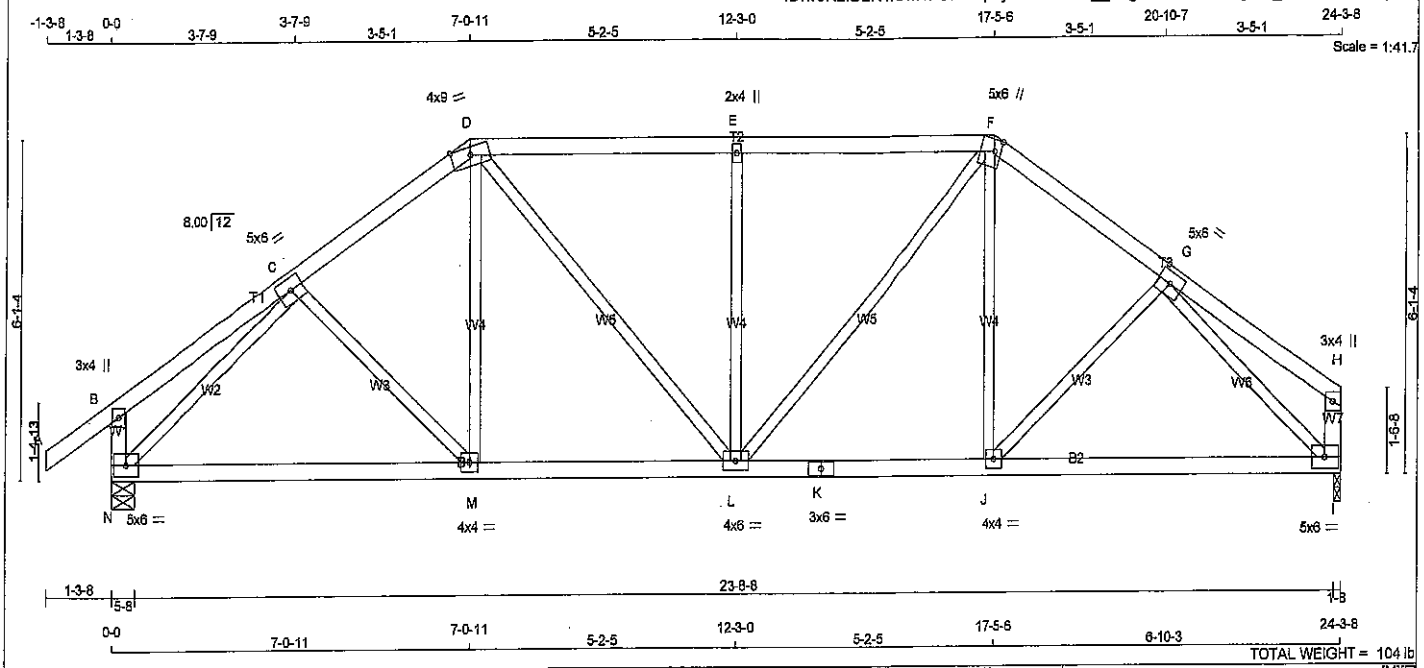


TOTAL WEIGHT = 104 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	8.0		
D	TTWW-m	MT20	4.0	9.0	Edge	
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	8.0	2.50	1.50
G	TMWW-t	MT20	5.0	8.0		
H	TMV+p	MT20	3.0	4.0		
I	BMWW-t	MT20	5.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	4.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMVW1-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
N	1479	0	1479	0	0	5-8	5-8
I	1363	0	1363	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1198	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0
I	1119	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0

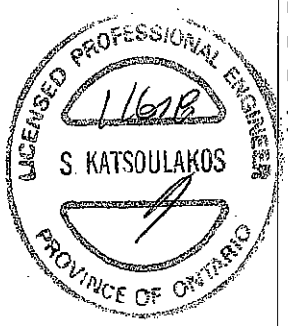
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.05 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	C- M	0 / 117	0.03 (3)	
B- C	0 / 17	-84.3	-84.3	0.15 (1)	10.00	M- D	0 / 305	0.07 (2)	
C- D	-1459 / 0	-84.3	-84.3	0.14 (1)	5.29	D- L	0 / 409	0.09 (1)	
D- E	-1474 / 0	-84.3	-84.3	0.31 (1)	5.05	L- E	-534 / 0	0.31 (1)	
E- F	-1474 / 0	-84.3	-84.3	0.31 (1)	5.05	L- F	0 / 442	0.10 (1)	
F- G	-1433 / 0	-84.3	-84.3	0.14 (1)	5.33	J- F	0 / 273	0.06 (2)	
G- H	0 / 17	-84.3	-84.3	0.14 (1)	10.00	J- G	0 / 138	0.03 (3)	
N- B	-233 / 0	0.0	0.0	0.02 (1)	7.81	N- C	-1669 / 0	0.64 (1)	
I- H	-108 / 0	0.0	0.0	0.01 (1)	7.81	G- I	-1635 / 0	0.59 (1)	
N- M	0 / 1196	-28.0	-28.0	0.45 (2)	10.00				
M- L	0 / 1201	-28.0	-28.0	0.46 (2)	10.00				
L- K	0 / 1179	-28.0	-28.0	0.45 (2)	10.00				
K- J	0 / 1179	-28.0	-28.0	0.45 (2)	10.00				
J- I	0 / 1136	-28.0	-28.0	0.43 (2)	10.00				



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.31/1.00 (D-E:1) , BC=0.46/1.00 (L-M:2) , WB=0.64/1.00 (C-N:1) , SSI=0.21/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (D) (INPUT = 0.90)
JSI METAL= 0.41 (C) (INPUT = 1.00)

DWG NO. TAM 4009-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285934	T8	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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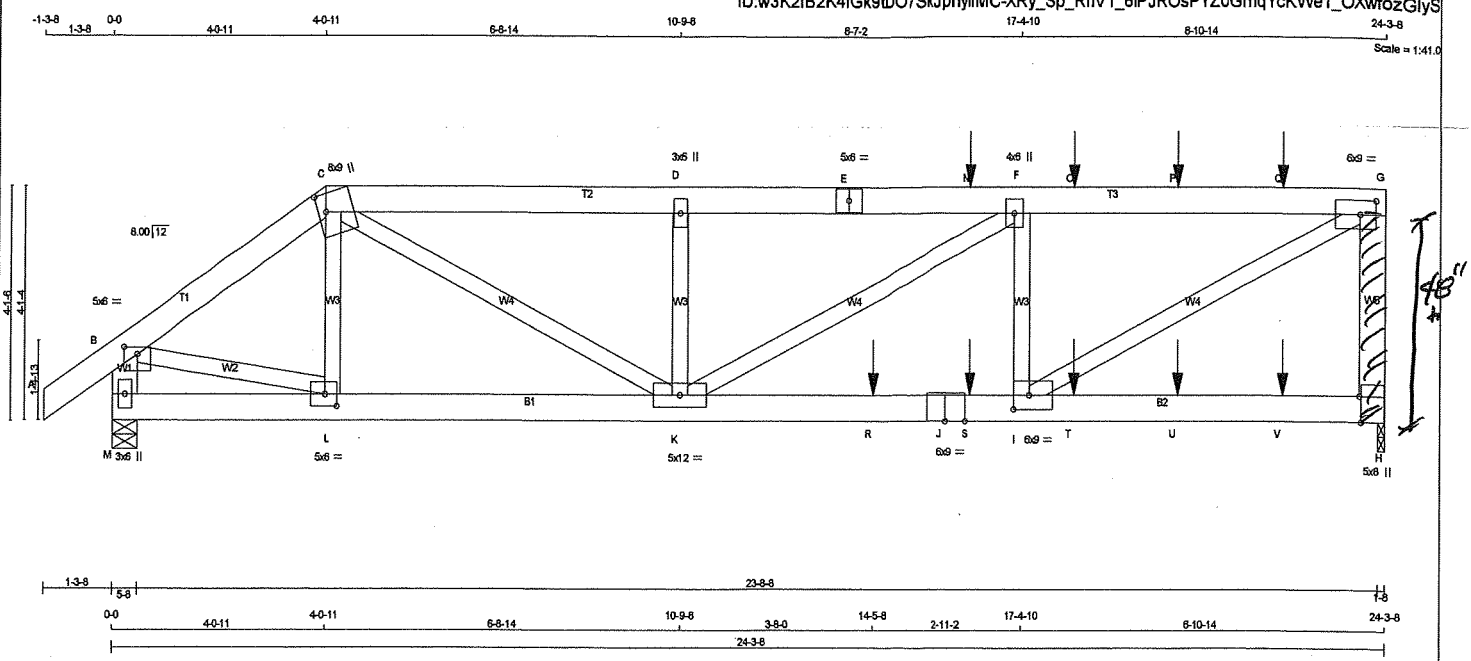

TOTAL WEIGHT = 138 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - G	2x6	DRY	No.2	SPF
H - G	2x6	DRY	No.2	SPF
M - B	2x6	DRY	No.2	SPF
M - J	2x6	DRY	2100F 1.8E	SPF
J - H	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TTWW+m	MT20	8.0	9.0	3.75	1.75
D	TMW+w	MT20	3.0	6.0		
E	TS-t	MT20	5.0	6.0		
F	TMWW+t	MT20	4.0	6.0		
G	TMVW-t	MT20	6.0	9.0	3.00	3.50
H	BMV1+p	MT20	5.0	8.0	Edge	0.25
I	BMWW-t	MT20	6.0	9.0	3.00	3.50
J	BS-t	MT20	6.0	9.0		
K	BMWWW-t	MT20	5.0	12.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.75
M	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1)

+ 48" VERTICAL SCAB

SCAB JOINT # H &# WITH 2 X 6 #2 SPF LONG 1 SIDE(S) USING 2 ROW(S) 3½" ARDOX SPIRAL/~~COMMON WIRE~~ NAILS AT 3" O/C STAGGERED (8 NAILS/SCAB). REQUIRED FOR 1½" BRG PROVISION.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	2719	0	2719	0	0	1-8	1-8
M	2212	0	2212	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	2211	1262 / 0	480 / 0	0 / 0	0 / 0	468 / 0	0 / 0
M	1781	1046 / 0	367 / 0	0 / 0	0 / 0	368 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.50 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 33	-84.3	-84.3	0.07 (1)	10.00	L- C	-437 / 17	0.07 (1)
B- C	-2504 / 0	-84.3	-84.3	0.17 (1)	5.05	B- L	0 / 2124	0.38 (1)
C- D	-4665 / 0	-84.3	-84.3	0.45 (1)	3.64	I- G	0 / 5007	0.89 (1)
D- E	-4665 / 0	-84.3	-84.3	0.55 (1)	3.50	C- K	0 / 2958	0.52 (1)
E- N	-4665 / 0	-84.3	-84.3	0.55 (1)	3.50	I- F	-1063 / 0	0.18 (1)
N- F	-4665 / 0	-84.3	-84.3	0.55 (1)	3.50	K- D	-564 / 0	0.10 (1)
F- O	-4428 / 0	-84.3	-84.3	0.61 (1)	3.50	K- F	0 / 272	0.05 (1)
O- P	-4428 / 0	-84.3	-84.3	0.61 (1)	3.50			
P- Q	-4428 / 0	-84.3	-84.3	0.61 (1)	3.50			
Q- G	-4428 / 0	-84.3	-84.3	0.61 (1)	3.50			
H- G	-2691 / 0	0.0	0.0	0.39 (1)	6.32			
M- B	-2131 / 0	0.0	0.0	0.15 (1)	6.93			
M- L	0 / 0	-28.0	-28.0	0.05 (1)	10.00			
L- K	0 / 2060	-28.0	-28.0	0.39 (1)	10.00			
K- R	0 / 4428	-28.0	-28.0	0.98 (1)	10.00			
R- J	0 / 4428	-28.0	-28.0	0.98 (1)	10.00			
J- S	0 / 4428	-28.0	-28.0	0.98 (1)	10.00			
S- I	0 / 4428	-28.0	-28.0	0.98 (1)	10.00			
I- T	0 / 0	-28.0	-28.0	0.33 (1)	10.00			
T- U	0 / 0	-28.0	-28.0	0.33 (1)	10.00			
U- V	0 / 0	-28.0	-28.0	0.33 (1)	10.00			
V- H	0 / 0	-28.0	-28.0	0.33 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
N	16-4-12	-69	-69	—	BACK	VERT	TOTAL
O	18-4-12	-69	-69	—	BACK	VERT	TOTAL
P	20-4-12	-69	-69	—	BACK	VERT	TOTAL
Q	22-4-12	-69	-69	—	BACK	VERT	TOTAL
R	14-5-8	-1197	-1197	—	BACK	VERT	TOTAL
S	16-4-12	-50	-50	—	BACK	VERT	TOTAL
T	18-4-12	-50	-50	—	BACK	VERT	TOTAL
U	20-4-12	-50	-50	—	BACK	VERT	TOTAL
V	22-4-12	-50	-50	—	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER 51518 S. KATSOULAKOS PROVINCE OF ONTARIO

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 968 (0.30")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 615 (0.47")

CSI: TC=0.61/1.00 (F-G:1) , BC=0.98/1.00 (I-K:1) , WB=0.89/1.00 (G-I:1) , SSI=0.66/1.00 (I-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (L) (INPUT = 0.90)
JSI METAL= 0.91 (J) (INPUT = 1.00)

CONTINUED ON PAGE 2

P6/4

DWG NO. TAM21154-18 RM
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285934	T8	1	1	TRUSS DESC.	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 101.4 lbs FACTORED DOWN AT 16-4-12, 101.4 lbs FACTORED DOWN AT 18-4-12, AND 101.4 lbs FACTORED DOWN AT 20-4-12, AND 101.4 lbs FACTORED DOWN AT 22-4-12 ON TOP CHORD, AND 1519.0 lbs FACTORED DOWN AT 14-5-8, 69.9 lbs FACTORED DOWN AT 16-4-12, 69.9 lbs FACTORED DOWN AT 18-4-12, AND 69.9 lbs FACTORED DOWN AT 20-4-12, AND 69.9 lbs FACTORED DOWN AT 22-4-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DWG NO. TAM 21154 -18 RH
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285994	T9	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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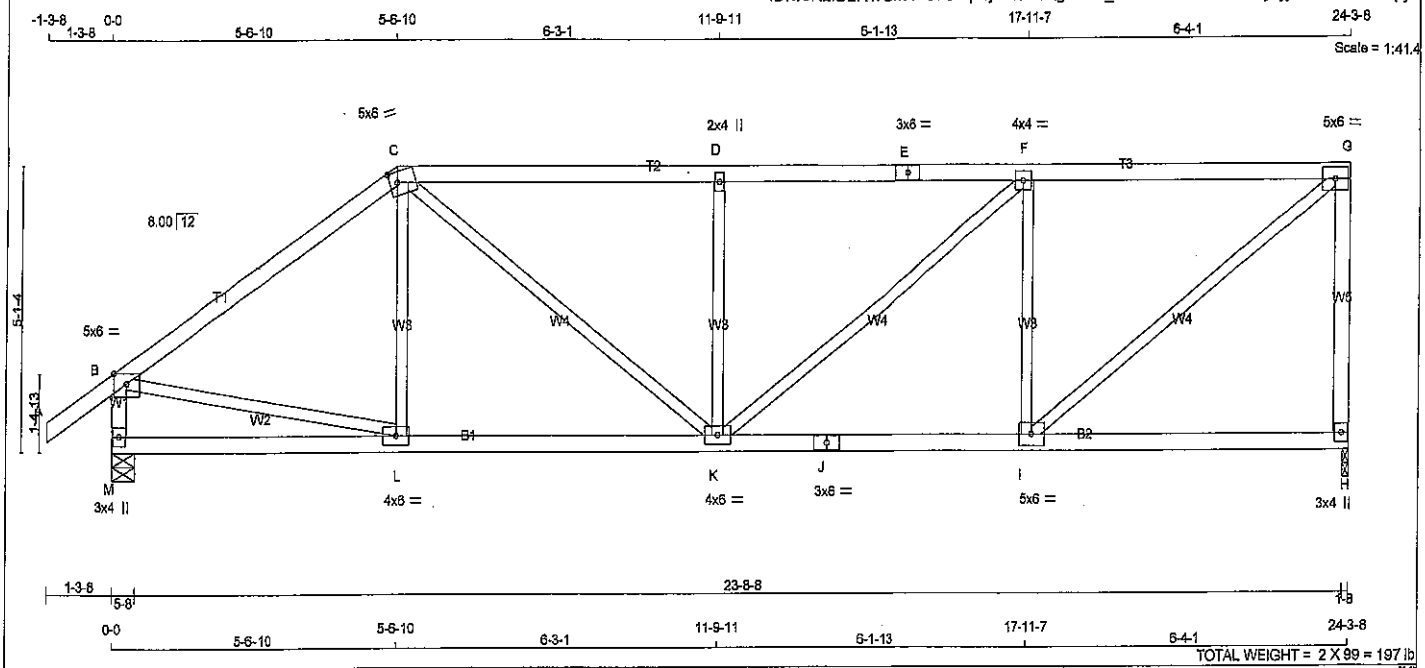

TOTAL WEIGHT = 2 X 99 = 197 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	5.0	6.0	Edge	
C	TTWW-m	MT20	5.0	6.0	2.25	1.75
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMWW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	1363	0	1363	0	0	1-8	1-8
M	1479	0	1479	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1119	821 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
M	1198	892 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.34 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	
B- C	-1493 / 0	-84.3	-84.3	0.55 (1)	4.89	
C- D	-1797 / 0	-84.3	-84.3	0.55 (1)	4.37	
D- E	-1797 / 0	-84.3	-84.3	0.56 (1)	4.34	
E- F	-1797 / 0	-84.3	-84.3	0.56 (1)	4.34	
F- G	-1416 / 0	-84.3	-84.3	0.53 (1)	4.81	
H- G	-1292 / 0	0.0	0.0	0.57 (1)	7.10	
M- B	-1417 / 0	0.0	0.0	0.15 (1)	6.85	
M- L	0 / 0	-28.0	-28.0	0.24 (3)	10.00	
L- K	0 / 1240	-28.0	-28.0	0.39 (2)	10.00	
K- J	0 / 1416	-28.0	-28.0	0.44 (2)	10.00	
J- I	0 / 1416	-28.0	-28.0	0.44 (2)	10.00	
I- H	0 / 0	-28.0	-28.0	0.27 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- C	-49 / 204	0.05 (3)
B- L	0 / 1263	0.28 (1)
I- G	0 / 1776	0.40 (1)
C- K	0 / 702	0.16 (1)
I- F	-877 / 0	0.34 (1)
K- D	-586 / 0	0.22 (1)
K- F	0 / 484	0.11 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.57/1.00 (G-H:1) , BC=0.44/1.00 (I-K:2) , WB=0.40/1.00 (G-I:1) , SSI=0.25/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.66 (K) (INPUT = 0.90)
JSI METAL= 0.44 (J) (INPUT = 1.00)

DWG NO. TAM 3488-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43654	DRWG NO.
285994	T10	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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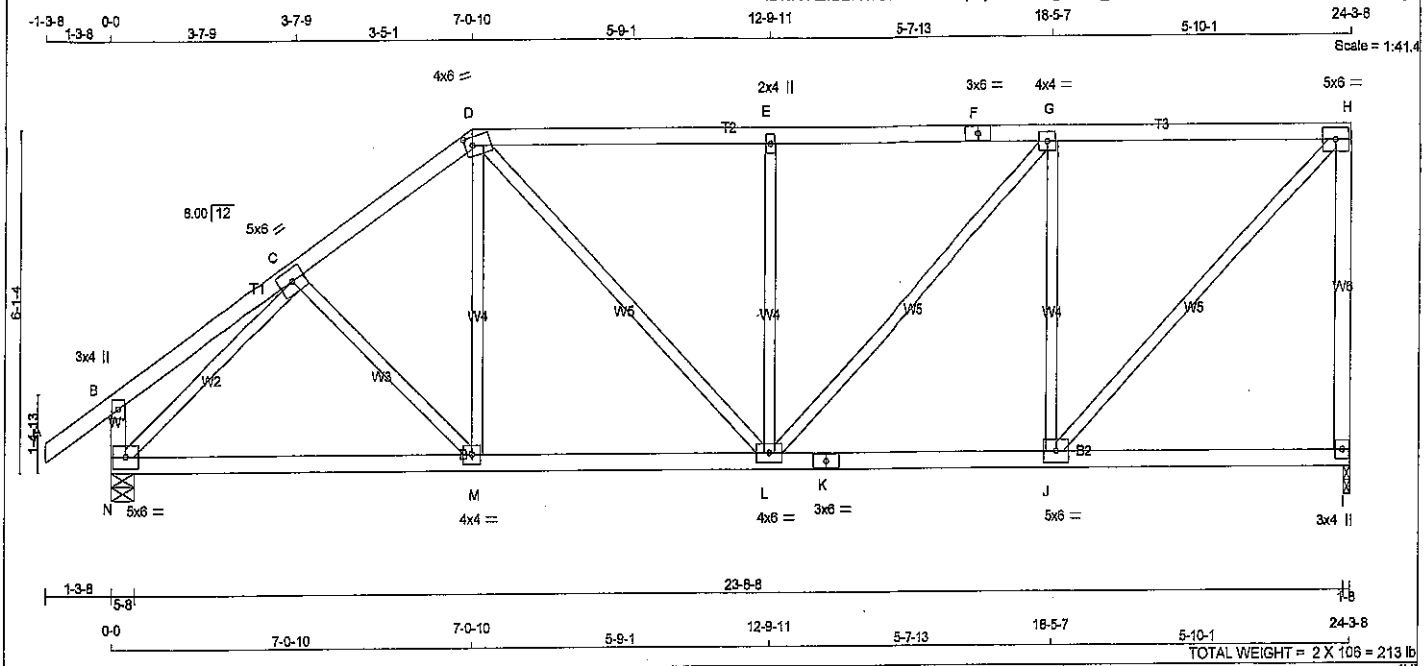


TOTAL WEIGHT = 2 X 106 = 213 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW-m	MT20	4.0	6.0	1.75	1.75
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TMVW-t	MT20	5.0	6.0		
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	5.0	6.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	4.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
I	1363	0	1363	0	0	1-8	1-8
N	1479	0	1479	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1119	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
N	1198	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	
B- C	0 / 17	-84.3	-84.3	0.15 (1)	10.00	
C- D	-1460 / 0	-84.3	-84.3	0.18 (1)	5.22	
D- E	-1473 / 0	-84.3	-84.3	0.43 (1)	4.86	
E- F	-1473 / 0	-84.3	-84.3	0.44 (1)	4.86	
F- G	-1473 / 0	-84.3	-84.3	0.44 (1)	4.86	
G- H	-1103 / 0	-84.3	-84.3	0.42 (1)	5.46	
I- H	-1298 / 0	0.0	0.0	0.94 (1)	7.08	
N- B	-233 / 0	0.0	0.0	0.02 (1)	7.81	
N- M	0 / 1196	-28.0	-28.0	0.45 (2)	10.00	
M- L	0 / 1202	-28.0	-28.0	0.47 (2)	10.00	
L- K	0 / 1103	-28.0	-28.0	0.37 (2)	10.00	
K- J	0 / 1103	-28.0	-28.0	0.37 (2)	10.00	
J- I	0 / 0	-28.0	-28.0	0.24 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- M	0 / 118	0.03 (3)
M- D	0 / 307	0.07 (2)
N- C	-1869 / 0	0.64 (1)
J- H	0 / 1557	0.35 (1)
D- L	0 / 385	0.09 (1)
J- G	-912 / 0	0.53 (1)
L- E	-520 / 0	0.30 (1)
L- G	0 / 530	0.12 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.94/1.00 (H-I:1) , BC=0.47/1.00 (L-M:2) , WB=0.64/1.00 (C-N:1) , SSI=0.23/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

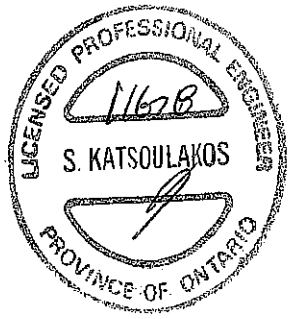
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (D) (INPUT = 0.90)
JSI METAL= 0.41 (C) (INPUT = 1.00)



DWG NO. TAM 3989 -18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285994	T11	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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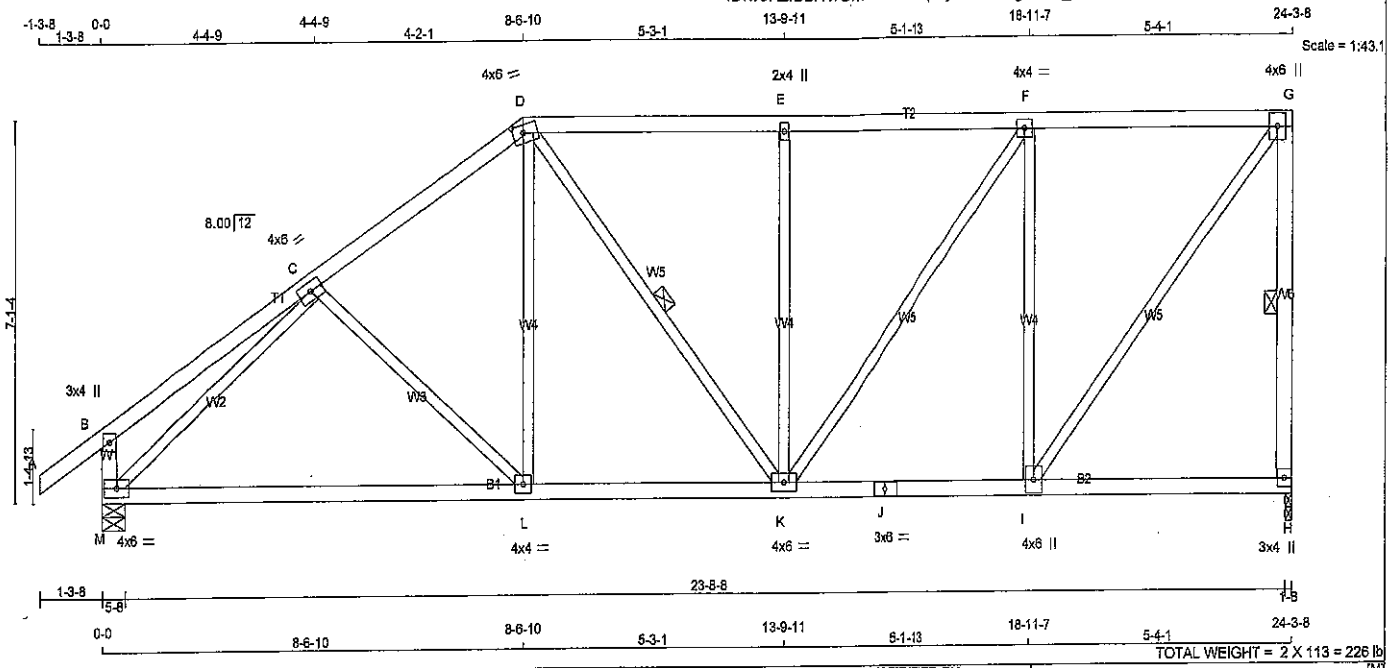


TOTAL WEIGHT = 2 X 113 = 226 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW-m	MT20	4.0	6.0	1.75	2.50
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMW+p	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWW+t	MT20	4.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1363	0	1363	0	0	1-8	1-8
M	1479	0	1479	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1119	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
M	1198	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

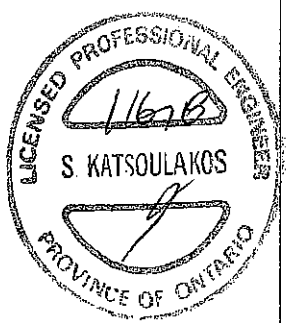
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-H, D-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 32	-84.3	-84.3	0.11	(1)	10.00	C- L	-117 / 85	0.07 (1)
B- C	0 / 23	-84.3	-84.3	0.24	(1)	10.00	L- D	0 / 414	0.09 (2)
C- D	-1390 / 0	-84.3	-84.3	0.28	(1)	5.20	M- C	-1661 / 0	0.93 (1)
D- E	-1226 / 0	-84.3	-84.3	0.34	(1)	5.37	I- G	0 / 1428	0.32 (1)
E- F	-1227 / 0	-84.3	-84.3	0.35	(1)	5.35	D- K	-6 / 141	0.03 (1)
F- G	-879 / 0	-84.3	-84.3	0.34	(1)	6.07	I- F	-946 / 0	0.83 (1)
H- G	-1305 / 0	0.0	0.0	0.30	(1)	5.66	K- E	-474 / 0	0.41 (1)
M- B	-255 / 0	0.0	0.0	0.03	(1)	7.81	K- F	0 / 576	0.13 (1)
M- L	0 / 1223	-28.0	-28.0	0.59	(2)	10.00			
L- K	0 / 1141	-28.0	-28.0	0.59	(2)	10.00			
K- J	0 / 879	-28.0	-28.0	0.31	(2)	10.00			
J- I	0 / 879	-28.0	-28.0	0.31	(2)	10.00			
I- H	0 / 0	-28.0	-28.0	0.21	(3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.23")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 779 (0.37")

CSI: TC=0.35/1.00 (E-F:1) , BC=0.59/1.00 (K-L:2) , WB=0.93/1.00 (C-M:1) , SSI=0.21/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (M) (INPUT = 0.90)
JSI METAL= 0.40 (C) (INPUT = 1.00)

DWG NO. TAM 3990 -13
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285994	T12	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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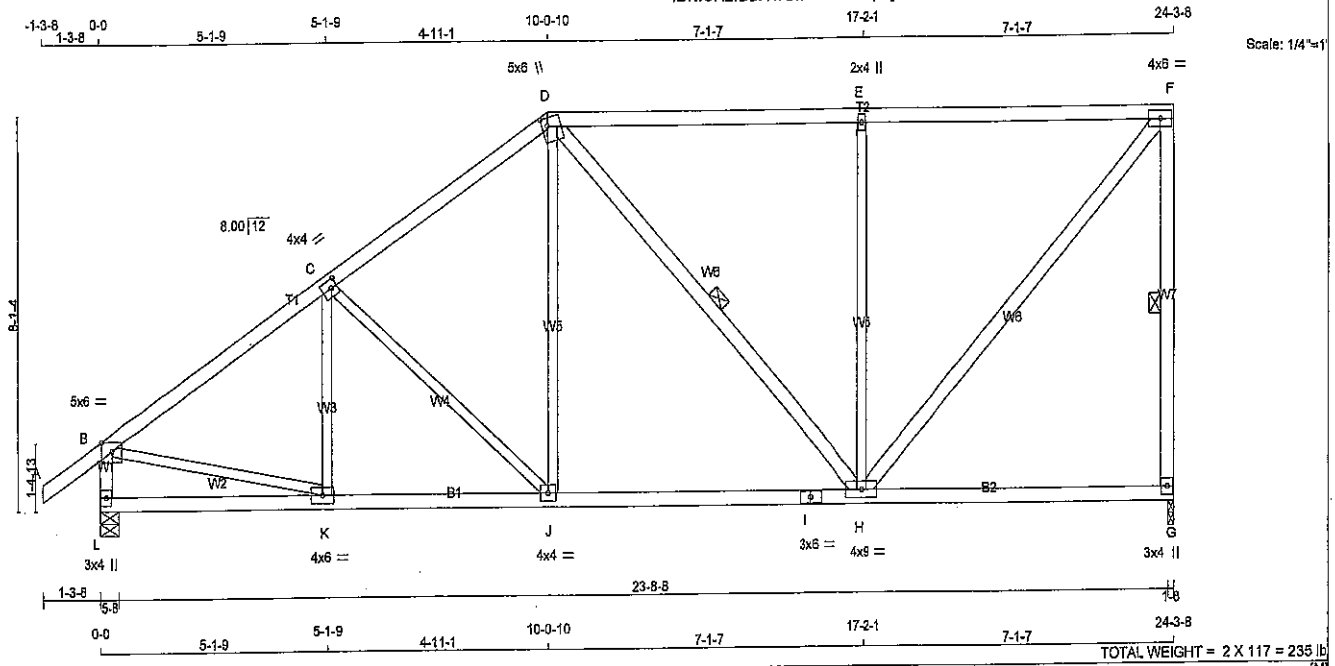

TOTAL WEIGHT = 2 X 117 = 235 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1363	0	1383	0	0	1-8	1-8
L	1479	0	1479	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1119	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
L	1198	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.81 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	K- C	-123 / 133	0.04 (1)	
B- C	-1529 / 0	-84.3	-84.3	0.43 (1)	4.83	C- J	-347 / 0	0.29 (1)	
C- D	-1275 / 0	-84.3	-84.3	0.40 (1)	5.20	J- D	0 / 436	0.10 (2)	
D- E	-987 / 0	-84.3	-84.3	0.80 (1)	4.81	D- H	-110 / 0	0.06 (1)	
E- F	-987 / 0	-84.3	-84.3	0.81 (1)	4.81	H- E	-745 / 0	0.84 (1)	
G- F	-1284 / 0	0.0	0.0	0.38 (1)	5.69	H- F	0 / 1432	0.23 (1)	
L- B	-1418 / 0	0.0	0.0	0.15 (1)	5.85	B- K	0 / 1323	0.30 (1)	
L- K	0 / 0	-28.0	-28.0	0.16 (3)	10.00				
K- J	0 / 1295	-28.0	-28.0	0.33 (2)	10.00				
J- I	0 / 1041	-28.0	-28.0	0.49 (2)	10.00				
I- H	0 / 1041	-28.0	-28.0	0.49 (2)	10.00				
H- G	0 / 0	-28.0	-28.0	0.37 (3)	10.00				

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.8	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.81/1.00 (E-F:1) , BC=0.49/1.00 (H-J:2) , WB=0.94/1.00 (E-H:1) , SSI=0.29/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL= 0.36 (I) (INPUT = 1.00)

DWG NO. TAM 3991-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
286039	T13	5	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Dec 12 2017 MiTek Industries, Inc. Tue Jan 16 10:38:29 2018 Page 1
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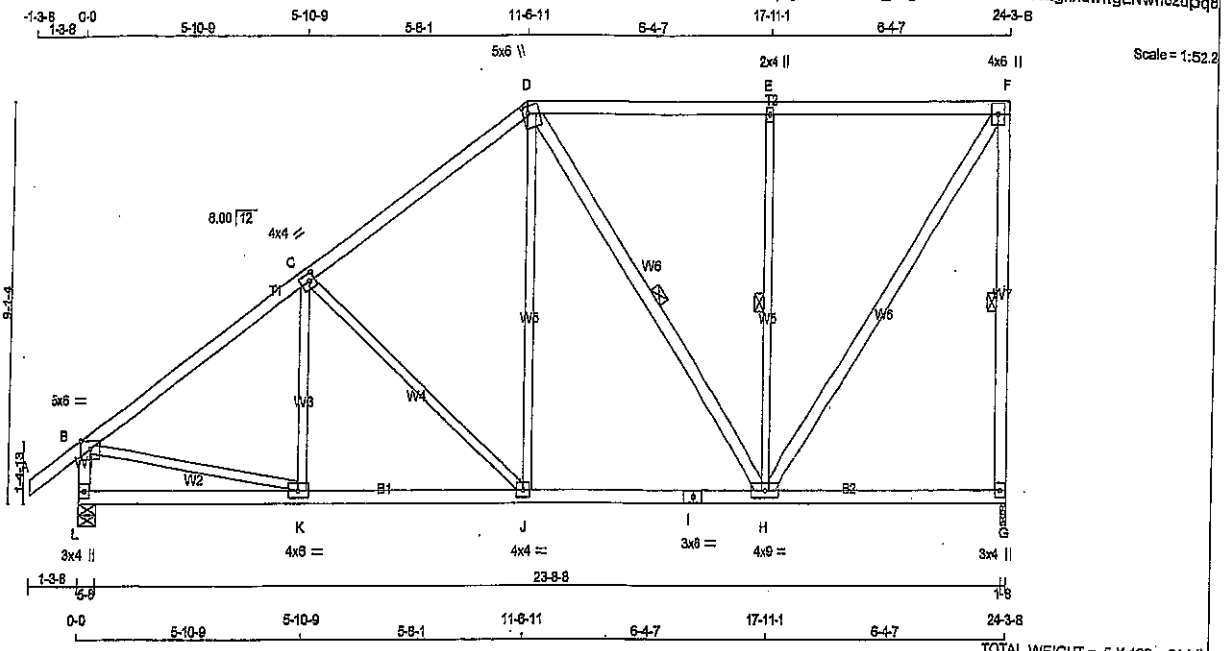

TOTAL WEIGHT = 5 X 123 = 614 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMWW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1363	0	1363	0	0	1-8	1-8
L	1479	0	1479	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1119	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
L	1196	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.61 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	K-C	-58 / 203	0.05 (3)
B-C	-1531 / 0	-84.3	-84.3	0.59 (1)	4.61	C-J	-474 / 0	0.56 (1)
C-D	-1167 / 0	-84.3	-84.3	0.54 (1)	5.16	J-D	0 / 490	0.11 (2)
D-E	-790 / 0	-84.3	-84.3	0.51 (1)	5.73	D-H	-270 / 0	0.17 (1)
E-F	-790 / 0	-84.3	-84.3	0.82 (1)	5.73	H-E	-666 / 0	0.36 (1)
G-F	-1292 / 0	0.0	0.0	0.50 (1)	5.68	H-F	0 / 1344	0.22 (1)
L-B	-1412 / 0	0.0	0.0	0.15 (1)	6.65	B-K	0 / 1322	0.30 (1)
L-K	0 / 0	-28.0	-28.0	0.23 (3)	10.00			
K-J	0 / 1300	-28.0	-28.0	0.38 (2)	10.00			
J-I	0 / 948	-28.0	-28.0	0.40 (2)	10.00			
I-H	0 / 948	-28.0	-28.0	0.40 (2)	10.00			
H-G	0 / 0	-28.0	-28.0	0.29 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.62/1.00 (E-F:1) , BC=0.40/1.00 (H-J:2) , WB=0.56/1.00 (C-J:1) , SSI=0.26/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.9D)
JSI METAL= 0.32 (K) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEERS
12678
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 3993 -13
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285349	T14-Cond1	6	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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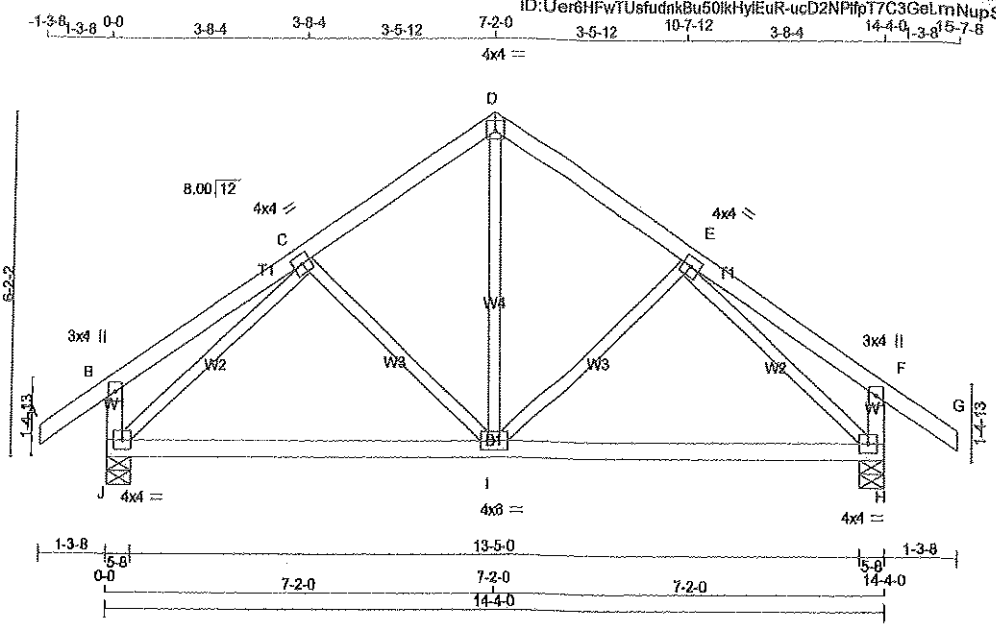

TOTAL WEIGHT = 6 X 62 = 371 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	920	0	920	0	0	5-8	5-8
H	920	0	920	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	739	437 / 0	151 / 0	0 / 0	0 / 0	152 / 0	0 / 0
H	739	437 / 0	151 / 0	0 / 0	0 / 0	152 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00
B- C	0 / 19	-84.3	-84.3	0.17 (1)	10.00
C- D	-645 / 0	-84.3	-84.3	0.13 (1)	6.25
D- E	-645 / 0	-84.3	-84.3	0.13 (1)	6.25
E- F	0 / 19	-84.3	-84.3	0.17 (1)	10.00
F- G	0 / 32	-84.3	-84.3	0.11 (1)	10.00
J- B	-232 / 0	0.0	0.0	0.02 (1)	7.81
H- F	-232 / 0	0.0	0.0	0.02 (1)	7.81
J- I	0 / 631	-28.0	-28.0	0.49 (2)	10.00
I- H	0 / 631	-28.0	-28.0	0.49 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- D	0 / 460	0.10 (1)
I- E	-152 / 44	0.06 (1)
C- I	-152 / 44	0.06 (1)
J- C	-679 / 0	0.35 (1)
E- H	-679 / 0	0.35 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.48")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.48")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.17/1.00 (E-F:1) , BC=0.49/1.00 (H-I:2) , WB=0.35/1.00 (E-H:1) , SSI=0.17/1.00 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS. BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1687	822	2284	1650

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (D) (INPUT = 0.90)
JSI METAL= 0.31 (C) (INPUT = 1.00)

DWG NO. TAM T1802642
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285349	T14-Cond2	6	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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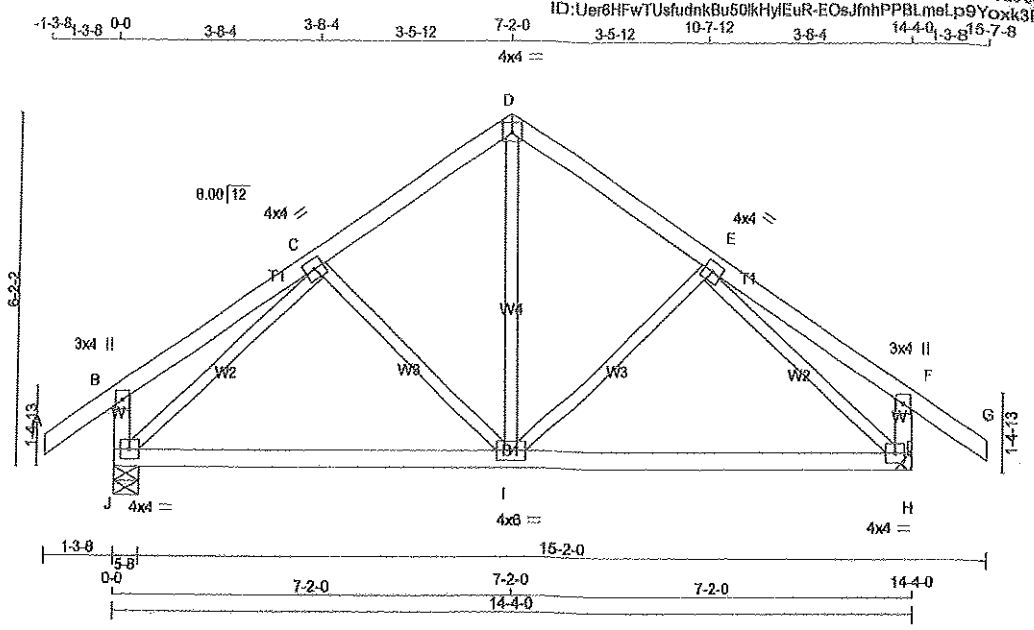


TOTAL WEIGHT = 6 X 62 = 371 lb [M|F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT	REQRD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG IN-SX	BRG IN-SX
J	920	0	920	0	0	5-8	5-8
H	920	0	920	0	0	MECHANICAL	

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT H.

UNFACTORED REACTIONS

	1ST LCASE	MAX /MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	739	437 / 0	151 / 0	0 / 0	0 / 0	152 / 0	0 / 0
H	739	437 / 0	151 / 0	0 / 0	0 / 0	152 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

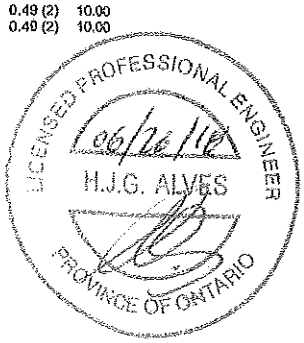
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00
B- C	0 / 19	-84.3	-84.3	0.17 (1)	10.00
C- D	-645 / 0	-84.3	-84.3	0.13 (1)	6.25
D- E	-645 / 0	-84.3	-84.3	0.13 (1)	6.25
E- F	0 / 19	-84.3	-84.3	0.17 (1)	10.00
F- G	0 / 32	-84.3	-84.3	0.11 (1)	10.00
J- B	-232 / 0	0.0	0.0	0.02 (1)	7.81
H- F	-232 / 0	0.0	0.0	0.02 (1)	7.81
J- I	0 / 631	-28.0	-28.0	0.49 (2)	10.00
I- H	0 / 631	-28.0	-28.0	0.49 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- D	0 / 460	0.10 (1)
I- E	-152 / 44	0.05 (1)
C- I	-152 / 44	0.03 (1)
J- C	-879 / 0	0.35 (1)
E- H	-879 / 0	0.35 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.48")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.48")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.17/1.00 (E-F:1) , BC=0.49/1.00 (H-I:2) , WB=0.35/1.00 (E-I:1) , SSI=0.17/1.00 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE REELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (D) (INPUT = 0.90)
JSI METAL= 0.31 (C) (INPUT = 1.00)

DWG NO. TAM 1800643
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285348	T24	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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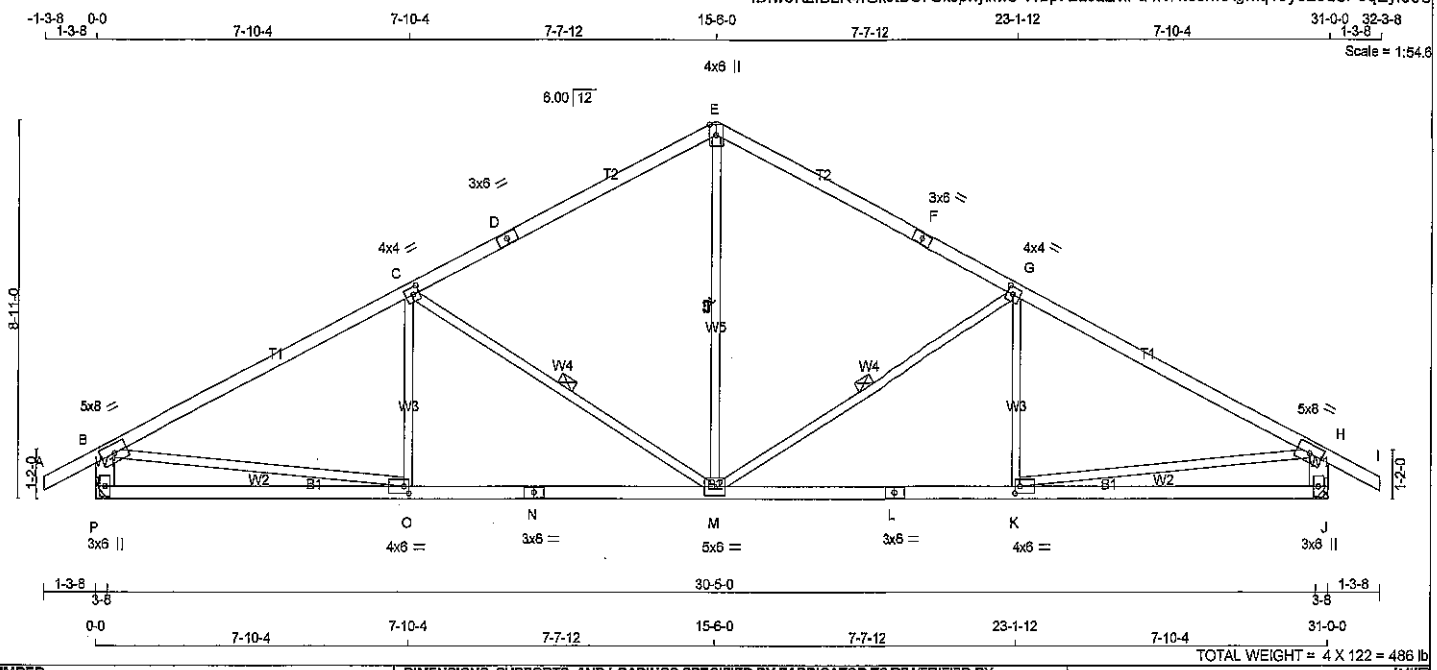


TOTAL WEIGHT = 4 X 122 = 486 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
P - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	6.0		
E	TTW+p	MT20	4.0	6.0	Edge	
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
H	TMVW-t	MT20	5.0	8.0		
J	BMV1+p	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	6.0	2.00	1.50
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	5.0	6.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	6.0	2.00	1.50
P	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	1854	0	1854	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	
J	1854	0	1854	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1506	863 / 0	326 / 0	0 / 0	0 / 0	316 / 0	0 / 0
J	1506	863 / 0	326 / 0	0 / 0	0 / 0	316 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.27 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-M, C-M. E-M

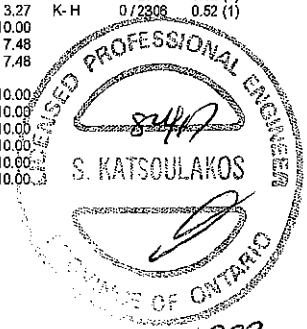
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 26	-84.3	-84.3	0.11 (1)	10.00
B- C	-2529 / 0	-84.3	-84.3	0.94 (1)	3.27
C- D	-1803 / 0	-84.3	-84.3	0.76 (1)	3.96
D- E	-1803 / 0	-84.3	-84.3	0.76 (1)	3.96
E- F	-1803 / 0	-84.3	-84.3	0.76 (1)	3.96
F- G	-1803 / 0	-84.3	-84.3	0.76 (1)	3.96
G- H	-2529 / 0	-84.3	-84.3	0.94 (1)	3.27
H- I	0 / 26	-84.3	-84.3	0.11 (1)	10.00
P- B	-1764 / 0	0.0	0.0	0.11 (1)	7.48
J- H	-1764 / 0	0.0	0.0	0.11 (1)	7.48
P- O	0 / 0	-28.0	-28.0	0.42 (3)	10.00
O- N	0 / 2293	-28.0	-28.0	0.68 (2)	10.00
N- M	0 / 2293	-28.0	-28.0	0.68 (2)	10.00
M- L	0 / 2293	-28.0	-28.0	0.68 (2)	10.00
L- K	0 / 2293	-28.0	-28.0	0.68 (2)	10.00
K- J	0 / 0	-28.0	-28.0	0.42 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- E	0 / 1096	0.25 (1)
M- G	-840 / 0	0.48 (1)
K- G	-9 / 307	0.07 (3)
C- M	-840 / 0	0.48 (1)
O- C	-9 / 307	0.07 (3)
B- O	0 / 2306	0.52 (1)
K- H	0 / 2306	0.52 (1)



DWG NO. TAM4/888-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.30")

CSI: TC=0.94 (G-H:1) , BC=0.68 (K-M:2) , WB=0.52 (H-K:1) , SSI=0.29 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2264	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (K) (INPUT = 0.90)
JSI METAL= 0.69 (L) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
286039	T37	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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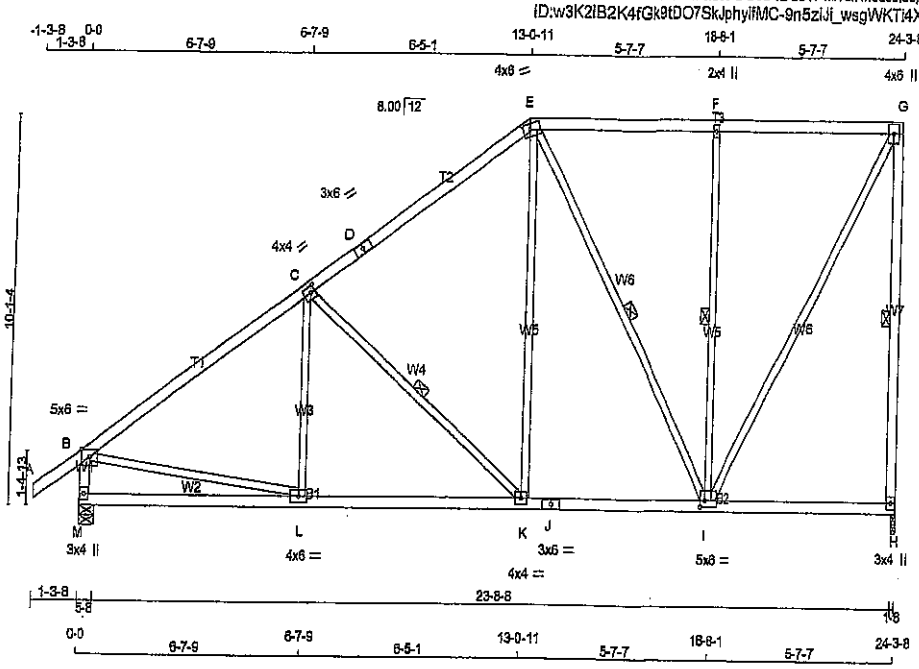

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	4.0	6.0	1.75	2.50
F	TMW+w	MT20	2.0	4.0		
G	TMWW+p	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWWW-t	MT20	5.0	6.0	2.00	1.50
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	1363	0	1363	0	0	1-8	1-8
M	1479	0	1479	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1119	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
M	1198	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.69 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-H, C-K, E-I, F-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.3	-84.3	1	0.11 (1)	10.00
B-C	-1522 / 0	-84.3	-84.3	1	0.55 (1)	4.89
C-D	-1051 / 0	-84.3	-84.3	1	0.49 (1)	5.46
D-E	-1051 / 0	-84.3	-84.3	1	0.49 (1)	5.46
E-F	-644 / 0	-84.3	-84.3	1	0.34 (1)	6.25
F-G	-644 / 0	-84.3	-84.3	1	0.34 (1)	6.25
H-G	-1299 / 0	0.0	0.0	1	0.55 (1)	5.66
M-B	-1404 / 0	0.0	0.0	1	0.14 (1)	6.87
M-L	0 / 0	-28.0	-28.0	1	0.31 (3)	10.00
L-K	0 / 1296	-28.0	-28.0	1	0.46 (2)	10.00
K-J	0 / 846	-28.0	-28.0	1	0.31 (2)	10.00
J-I	0 / 846	-28.0	-28.0	1	0.31 (2)	10.00
I-H	0 / 0	-28.0	-28.0	1	0.21 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L-C	-1 / 267	0.06 (3)
C-K	-594 / 0	0.30 (1)
K-E	0 / 545	0.12 (2)
E-I	-413 / 0	0.40 (1)
I-F	-586 / 0	0.41 (1)
I-G	0 / 1289	0.29 (1)
B-L	0 / 1314	0.30 (1)

TOTAL WEIGHT = 2 X 122 = 243 lb

[M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.8	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.65/1.00 (G-H:1) , BC=0.46/1.00 (K-L:2) , WB=0.41/1.00 (F-I:1) , SSI=0.23/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.83 (L) (INPUT = 0.90)
SI METAL= 0.32 (L) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM3994 -12
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285956	T40	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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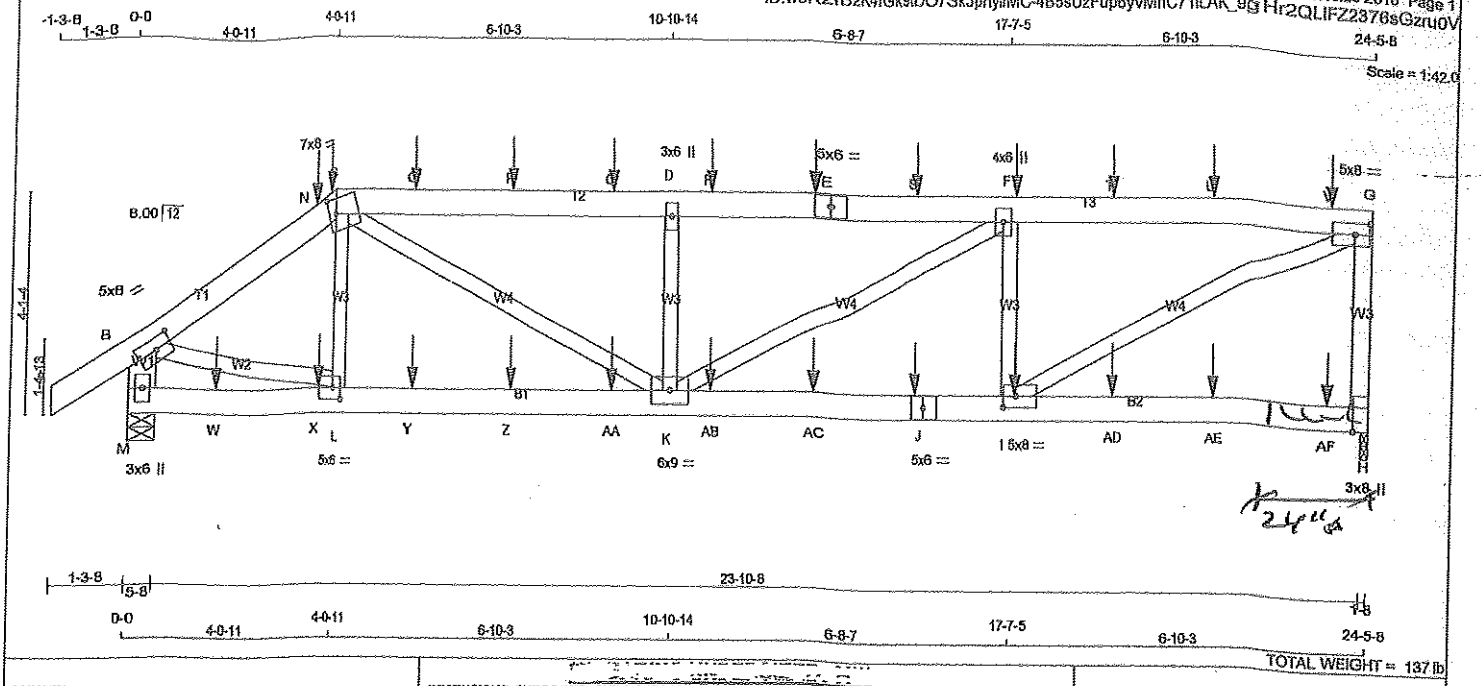


TOTAL WEIGHT = 137 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - G	2x6	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.50	3.75
C	TTWW-m	MT20	7.0	8.0	3.25	2.00
D	TMW+w	MT20	3.0	6.0		
E	TS-t	MT20	5.0	6.0		
F	TMWW+t	MT20	4.0	8.0		
G	TMVW-t	MT20	5.0	8.0	2.50	3.00
H	BMV1+p	MT20	3.0	8.0	Edge	0.50
I	BMWW-t	MT20	5.0	8.0	2.50	3.00
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	6.0	9.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.25
M	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 244.5 lbs FACTORED DOWN AT 4-0-11, 129.8 lbs FACTORED DOWN AT 3-8-12, 101.4 lbs FACTORED DOWN AT 5-8-12, 101.4 lbs FACTORED DOWN AT 7-8-12, 101.4 lbs FACTORED DOWN AT 9-8-12, 101.4 lbs FACTORED DOWN AT 11-8-12, 101.4 lbs FACTORED DOWN AT 13-8-12, 101.4 lbs FACTORED DOWN AT 15-8-12, 101.4 lbs FACTORED DOWN AT 17-8-12, 101.4 lbs FACTORED DOWN AT 19-8-12, AND 101.4 lbs FACTORED DOWN AT 21-8-12, AND 112.8 lbs FACTORED DOWN AT 23-8-12 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 1-8-12, 69.9 lbs FACTORED DOWN AT 3-8-12, 69.9 lbs FACTORED DOWN AT 5-8-12, 69.9 lbs FACTORED DOWN AT 7-8-12, 69.9 lbs FACTORED DOWN AT 9-8-12, 69.9 lbs FACTORED DOWN AT 11-8-12, 69.9 lbs FACTORED DOWN AT 13-8-12, 69.9 lbs FACTORED DOWN AT 15-8-12, 69.9 lbs FACTORED DOWN AT 17-8-12, 69.9 lbs FACTORED DOWN AT 19-8-12, AND 69.9 lbs FACTORED DOWN AT 21-8-12, AND 76.6 lbs FACTORED DOWN AT 23-8-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	2309	0	2309	0	0	1-8	1-8
M	2439	0	2439	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1907	1040 / 0	447 / 0	0 / 0	0 / 0	420 / 0	0 / 0
M	1982	1134 / 0	429 / 0	0 / 0	0 / 0	420 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.72 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (1)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 33	-84.3	-84.3	0.07 (1)	10.00
B-N	-2810 / 0	-84.3	-84.3	0.19 (1)	4.80
N-C	-2810 / 0	-84.3	-84.3	0.19 (1)	4.80
C-O	-4018 / 0	-84.3	-84.3	0.56 (1)	3.72
O-P	-4018 / 0	-84.3	-84.3	0.56 (1)	3.72
P-Q	-4018 / 0	-84.3	-84.3	0.55 (1)	3.72
Q-D	-4018 / 0	-84.3	-84.3	0.56 (1)	3.72
D-R	-4018 / 0	-84.3	-84.3	0.55 (1)	3.72
R-E	-4018 / 0	-84.3	-84.3	0.55 (1)	3.72
E-S	-4018 / 0	-84.3	-84.3	0.55 (1)	3.72
S-F	-4018 / 0	-84.3	-84.3	0.55 (1)	3.72
F-T	-3301 / 0	-84.3	-84.3	0.53 (1)	4.08
T-U	-3301 / 0	-84.3	-84.3	0.53 (1)	4.08
U-V	-3301 / 0	-84.3	-84.3	0.53 (1)	4.08
V-G	-3301 / 0	-84.3	-84.3	0.53 (1)	4.08
H-G	-2160 / 0	0.0	0.0	0.50 (1)	5.72
M-B	-2368 / 0	0.0	0.0	0.17 (1)	6.62
M-W	0 / 0	-28.0	-28.0	0.19 (2)	10.00
W-X	0 / 0	-28.0	-28.0	0.19 (2)	10.00
X-L	0 / 0	-28.0	-28.0	0.19 (2)	10.00
L-Y	0 / 2349	-28.0	-28.0	0.42 (1)	10.00
Y-Z	0 / 2349	-28.0	-28.0	0.42 (1)	10.00
Z-AA	0 / 2349	-28.0	-28.0	0.42 (1)	10.00
AA-K	0 / 2349	-28.0	-28.0	0.42 (1)	10.00
K-AB	0 / 3301	-28.0	-28.0	0.57 (2)	10.00
AB-AC	0 / 3301	-28.0	-28.0	0.57 (2)	10.00
AC-J	0 / 3301	-28.0	-28.0	0.57 (2)	10.00
J-I	0 / 3301	-28.0	-28.0	0.57 (2)	10.00
I-AD	0 / 0	-28.0	-28.0	0.28 (3)	10.00
AD-AE	0 / 0	-28.0	-28.0	0.26 (3)	10.00
AE-AF	0 / 0	-28.0	-28.0	0.26 (3)	10.00
AF-H	0 / 0	-28.0	-28.0	0.26 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L-C	-197 / 265	0.05 (3)
C-K	0 / 1888	0.33 (1)
K-D	-972 / 0	0.17 (1)
K-F	0 / 816	0.14 (1)
I-F	-1395 / 0	0.24 (1)
I-G	0 / 3739	0.66 (1)
B-L	0 / 2410	0.43 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	4-0-11	-23	-25	---	FRONT	VERT	DEAD
C	4-0-11	-222	-222	---	FRONT	VERT	SNOW
E	13-8-12	-101	-101	---	BACK	VERT	TOTAL

DESIGN CRITERIA [M]

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.62")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.82")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.56/1.00 (C-D:1) , BC=0.57/1.00 (I-K:2) , WB=0.66/1.00 (G-I:1) , SSI=0.35/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

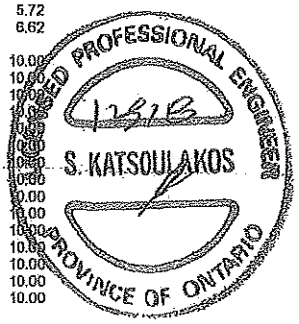
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.62 (G) (INPUT = 1.00)



DWG NO. TAM-5982-15
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285956	T40	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
F	17-8-12	-101	-101	--	BACK	VERT	TOTAL
I	17-8-12	-40	-70	--	BACK	VERT	TOTAL
J	15-8-12	-40	-70	--	BACK	VERT	TOTAL
N	3-8-12	-130	-130	--	BACK	VERT	TOTAL
O	5-8-12	-101	-101	--	BACK	VERT	TOTAL
P	7-8-12	-101	-101	--	BACK	VERT	TOTAL
Q	9-8-12	-101	-101	--	BACK	VERT	TOTAL
R	11-8-12	-101	-101	--	BACK	VERT	TOTAL
S	15-8-12	-101	-101	--	BACK	VERT	TOTAL
T	19-8-12	-101	-101	--	BACK	VERT	TOTAL
U	21-8-12	-101	-101	--	BACK	VERT	TOTAL
V	23-8-12	-113	-113	--	BACK	VERT	TOTAL
W	1-8-12	-40	-70	--	BACK	VERT	TOTAL
X	3-8-12	-40	-70	--	BACK	VERT	TOTAL
Y	5-8-12	-40	-70	--	BACK	VERT	TOTAL
Z	7-8-12	-40	-70	--	BACK	VERT	TOTAL
AA	9-8-12	-40	-70	--	BACK	VERT	TOTAL
AB	11-8-12	-40	-70	--	BACK	VERT	TOTAL
AC	13-8-12	-40	-70	--	BACK	VERT	TOTAL
AD	19-8-12	-40	-70	--	BACK	VERT	TOTAL
AE	21-8-12	-40	-70	--	BACK	VERT	TOTAL
AF	23-8-12	-44	-77	--	BACK	VERT	TOTAL

SCAB JOINT # H & G WITH 2 X 6 #2
SPF 24" LONG 1 SIDE(S) USING 2
ROW(S) 3/4 " COMMON WIRE NAILS
AT 2 " O/C STAGGERED (18 NAILS/SCAB)
REQUIRED FOR 1.15 BRG PROVISION.

LICENSED PROFESSIONAL ENGINEER
12/1/18
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 5902-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285815	T41	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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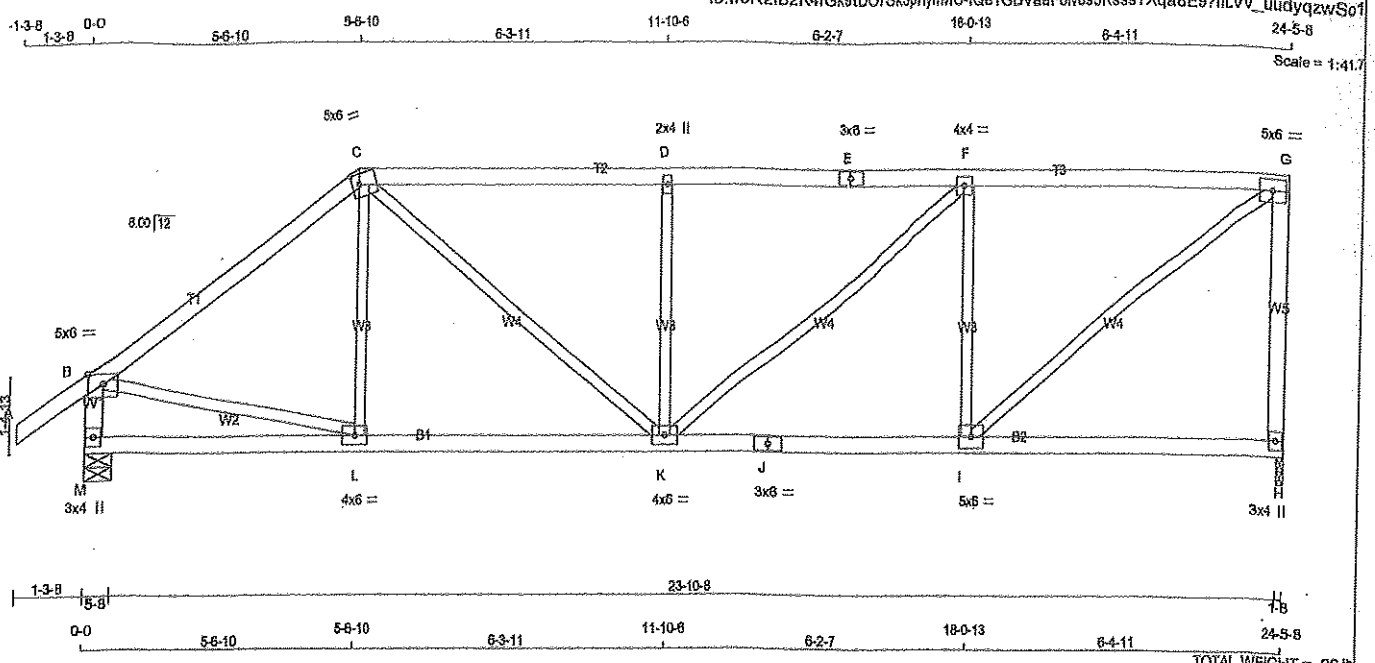

TOTAL WEIGHT = 99 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW-p	MT20	5.0	6.0	Edge	
C	TTWW-m	MT20	5.0	6.0	2.25	1.75
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	1373	0	1373	0	0	1-8	1-8
M	1486	0	1486	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1127	626 / 0	257 / 0	0 / 0	0 / 0	245 / 0	0 / 0
M	1206	698 / 0	257 / 0	0 / 0	0 / 0	253 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.30 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	
B- C	-1507 / 0	-84.3	-84.3	0.55 (1)	4.68	
C- D	-1822 / 0	-84.3	-84.3	0.56 (1)	4.33	
D- E	-1822 / 0	-84.3	-84.3	0.56 (1)	4.30	
E- F	-1822 / 0	-84.3	-84.3	0.56 (1)	4.30	
F- G	-1437 / 0	-84.3	-84.3	0.54 (1)	4.77	
H- G	-1360 / 0	0.0	0.0	0.57 (1)	7.08	
M- B	-1427 / 0	0.0	0.0	0.15 (1)	6.84	
M- L	0 / 0	-28.0	-28.0	0.24 (3)	10.00	
L- K	0 / 1251	-28.0	-28.0	0.39 (2)	10.00	
K- J	0 / 1437	-28.0	-28.0	0.44 (2)	10.00	
J- I	0 / 1437	-28.0	-28.0	0.44 (2)	10.00	
I- H	0 / 0	-28.0	-28.0	0.28 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- C	-50 / 204	0.05 (3)
B- L	0 / 1275	0.29 (1)
I- G	0 / 1799	0.40 (1)
C- K	0 / 717	0.16 (1)
I- F	-682 / 0	0.34 (1)
K- D	-571 / 0	0.22 (1)
K- F	0 / 487	0.11 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.82")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.82")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.58/1.00 (D-F:1) , BC=0.44/1.00 (I-K:2) , WB=0.40/1.00 (G-I:1) , SSI=0.25/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1697	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL= 0.44 (J) (INPUT = 1.00)

DWG NO. PAM3363.18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285815	T41A	1	1	TRUSS DESC.	

Tamarack Roof Trusses, Burlington

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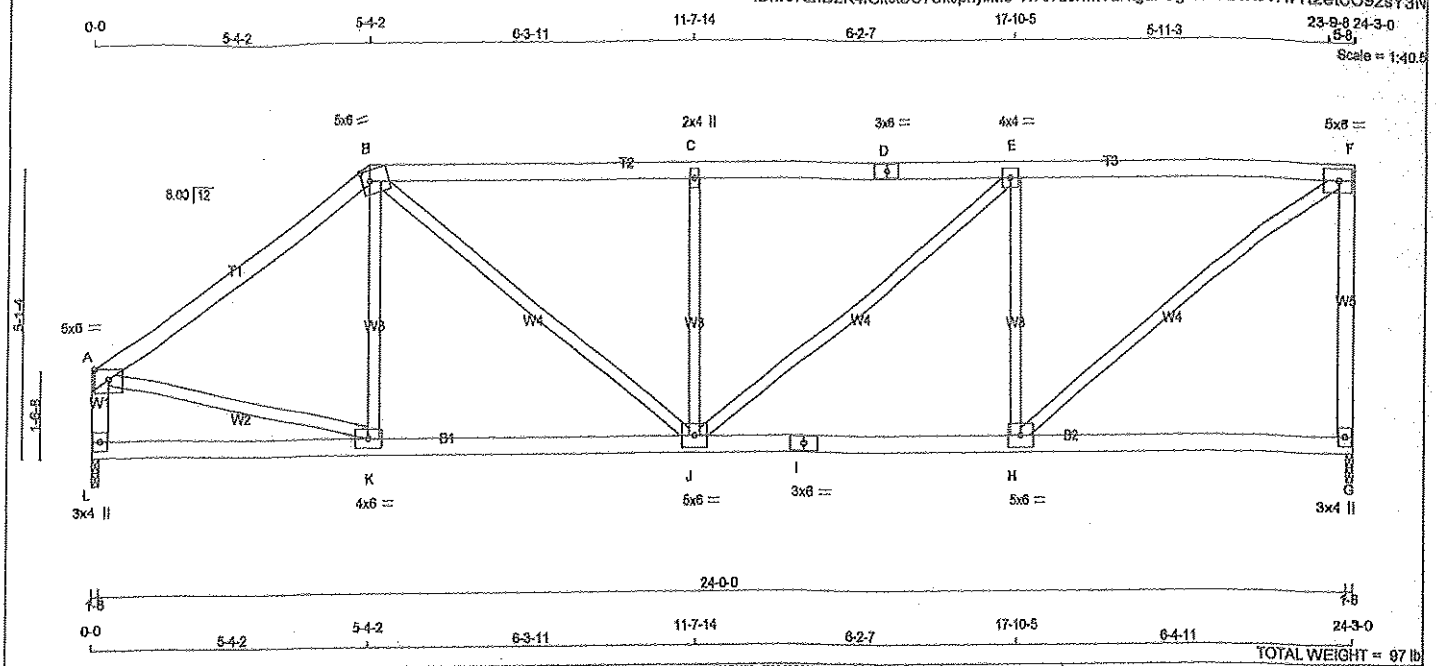

TOTAL WEIGHT = 97 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - A	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	6.0	Edge	
B	TTWW-m	MT20	5.0	6.0	2.25	1.75
C	TMW+w	MT20	2.0	4.0		
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMVW-t	MT20	5.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	5.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	5.0	6.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1381	0	1381	0	0	1-8	1-8
L	1381	0	1381	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1116	620 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
L	1116	620 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.33 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	-1451 / 0	-84.3	-84.3	0.50 (1)	4.82	K-B	-85 / 161	0.04 (3)
B-C	-1791 / 0	-84.3	-84.3	0.58 (1)	4.36	A-K	0 / 1236	0.28 (1)
C-D	-1792 / 0	-84.3	-84.3	0.57 (1)	4.33	H-F	0 / 1780	0.40 (1)
D-E	-1792 / 0	-84.3	-84.3	0.57 (1)	4.33	B-J	0 / 738	0.17 (1)
E-F	-1422 / 0	-84.3	-84.3	0.54 (1)	4.79	H-E	-870 / 0	0.33 (1)
G-F	-1289 / 0	0.0	0.0	0.57 (1)	7.10	J-C	-571 / 0	0.22 (1)
L-A	-1302 / 0	0.0	0.0	0.14 (1)	7.08	J-E	0 / 468	0.11 (1)
L-K	0 / 0	-28.0	-28.0	0.23 (3)	10.00			
K-J	0 / 1204	-28.0	-28.0	0.36 (2)	10.00			
J-I	0 / 1422	-28.0	-28.0	0.44 (2)	10.00			
I-H	0 / 1422	-28.0	-28.0	0.44 (2)	10.00			
H-G	0 / 0	-28.0	-28.0	0.28 (3)	10.00			

DESIGN CRITERIA [M]

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	48.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.57/1.00 (C-E:1) , BC=0.44/1.00 (H-J:2) , WB=0.40/1.00 (F-H:1) , SSI=0.25/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (K) (INPUT = 0.50)
JSI METAL= 0.44 (I) (INPUT = 1.00)

DWG NO. TAM 5072
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285815	T42A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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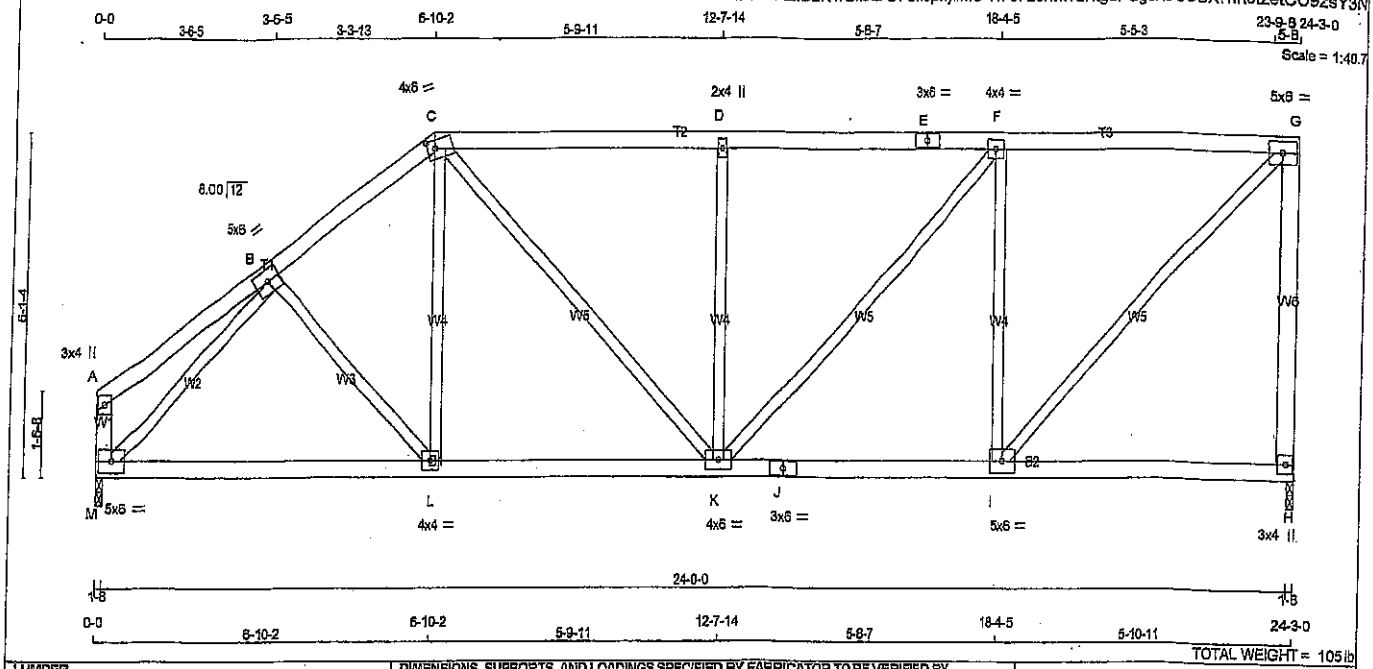

TOTAL WEIGHT = 105 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - A	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	5.0	6.0		
C	TTWW-m	MT20	4.0	6.0	1.75	1.75
D	TMV+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1361	0	1361	0	0	1-8	1-8
M	1361	0	1361	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1118	620 / 0	255 / 0	0 / 0	0 / 0	242 / 0	0 / 0
M	1118	620 / 0	255 / 0	0 / 0	0 / 0	242 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.85 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 17	-84.3 -84.3	0.14 (1)	10.00	B-L	0 / 134	0.03 (3)
B-C	-1430 / 0	-84.3 -84.3	0.17 (1)	5.28	L-C	0 / 279	0.06 (2)
C-D	-1471 / 0	-84.3 -84.3	0.44 (1)	4.85	M-B	-1633 / 0	0.62 (1)
D-E	-1471 / 0	-84.3 -84.3	0.45 (1)	4.85	I-G	0 / 1558	0.35 (1)
E-F	-1471 / 0	-84.3 -84.3	0.45 (1)	4.85	C-K	0 / 415	0.09 (1)
F-G	-1109 / 0	-84.3 -84.3	0.43 (1)	5.43	I-F	-908 / 0	0.53 (1)
H-G	-1295 / 0	0.0 0.0	0.93 (1)	7.10	K-D	-525 / 0	0.31 (1)
M-A	-114 / 0	0.0 0.0	0.01 (1)	7.81	K-F	0 / 517	0.12 (1)
M-L	0 / 1141	-28.0 -28.0	0.43 (2)	10.00			
L-K	0 / 1177	-28.0 -28.0	0.45 (2)	10.00			
K-J	0 / 1109	-28.0 -28.0	0.37 (2)	10.00			
J-I	0 / 1109	-28.0 -28.0	0.37 (2)	10.00			
I-H	0 / 0	-28.0 -28.0	0.24 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.93/1.00 (G-H:1) , BC=0.45/1.00 (K-L:2) , WB=0.62/1.00 (B-M:1) , SSI=0.23/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	522	2284	1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.86 (C) (INPUT = 0.90)
JSI METAL= 0.40 (B) (INPUT = 1.00)

DWG NO. TAM 5073 -15
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ORWG NO.
285815	T43A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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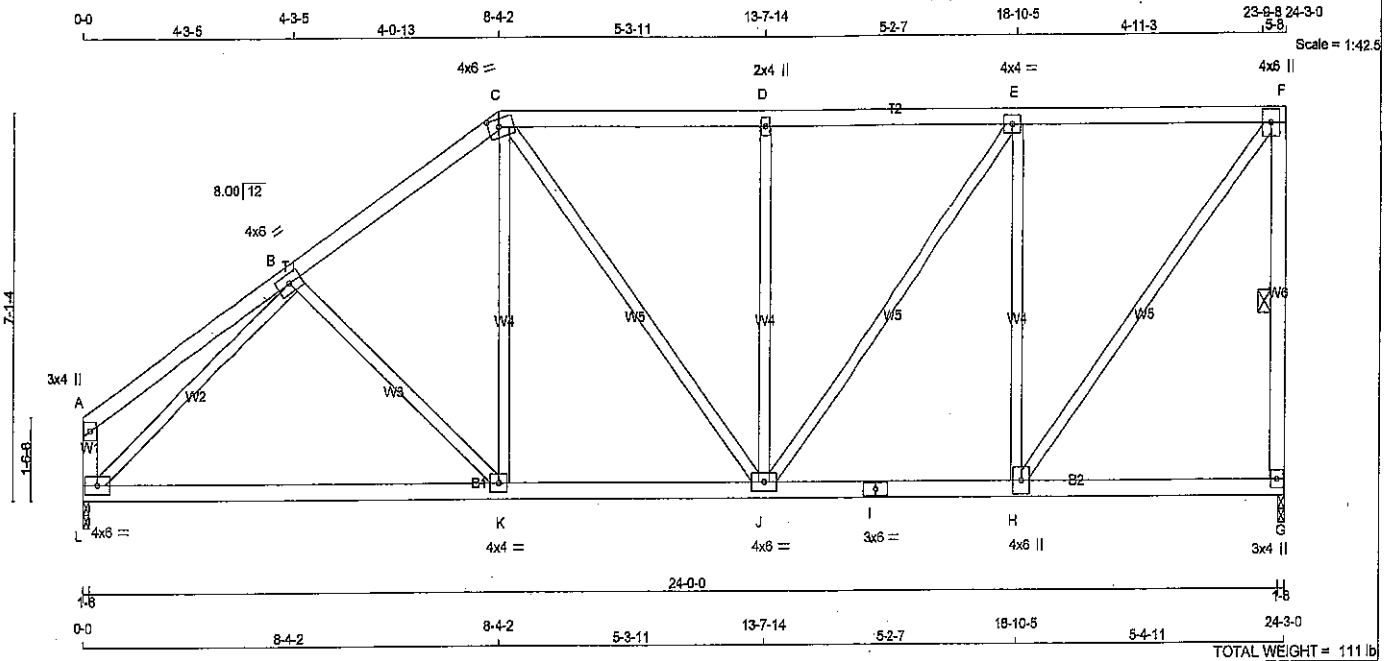

TOTAL WEIGHT = 111 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - A	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	6.0		
C	TTWW-m	MT20	4.0	6.0	1.75	2.50
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMWW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWW+t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1361	0	1361	0	0	1-8	1-8
L	1361	0	1361	0	0	1-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1118	620 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
L	1118	620 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 22	-84.3	-84.3	0.23 (1)	10.00	
B- C	-1369 / 0	-84.3	-84.3	0.27 (1)	5.25	
C- D	-1227 / 0	-84.3	-84.3	0.35 (1)	5.36	
D- E	-1227 / 0	-84.3	-84.3	0.38 (1)	5.33	
E- F	-884 / 0	-84.3	-84.3	0.34 (1)	6.05	
G- F	-1302 / 0	0.0	0.0	0.30 (1)	5.86	
L- A	-136 / 0	0.0	0.0	0.01 (1)	7.81	
L- K	0 / 1176	-28.0	-28.0	0.56 (2)	10.00	
K- J	0 / 1124	-28.0	-28.0	0.57 (2)	10.00	
J- I	0 / 884	-28.0	-28.0	0.32 (2)	10.00	
I- H	0 / 884	-28.0	-28.0	0.32 (2)	10.00	
H- G	0 / 0	-28.0	-28.0	0.21 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- K	-77 / 99	0.05 (1)
K- C	0 / 387	0.09 (2)
L- B	-1629 / 0	0.90 (1)
H- F	0 / 1425	0.32 (1)
C- J	0 / 168	0.04 (1)
H- E	-940 / 0	0.82 (1)
J- D	-479 / 0	0.42 (1)
J- E	0 / 565	0.13 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.21")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 853 (0.34")

CSI: TC=0.36/1.00 (D-E:1) , BC=0.57/1.00 (J-K:2) , WB=0.90/1.00 (B-L:1) , SSI=0.21/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.40 (B) (INPUT = 1.00)

DWG NO. TAM5074 -B
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285815	T44A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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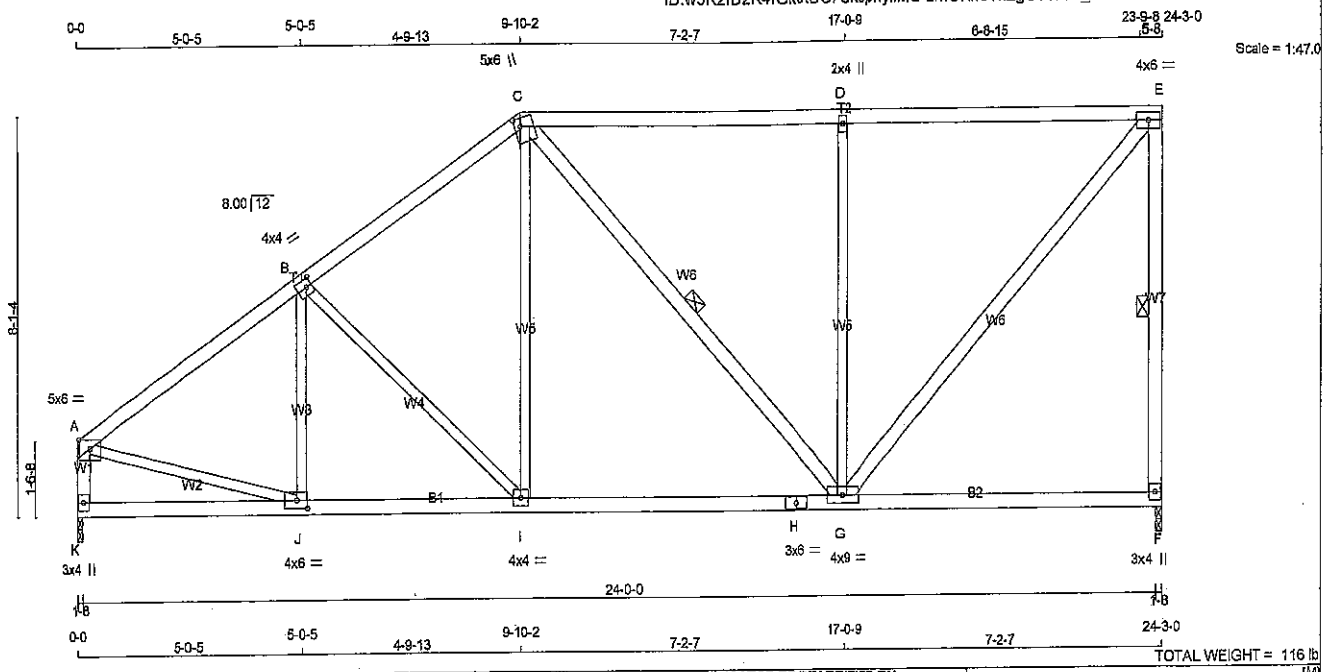

TOTAL WEIGHT = 116 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	6.0	Edge	
B	TMWW-t	MT20	4.0	4.0	2.00	1.50
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMWW-t	MT20	4.0	6.0	2.00	2.75
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	1381	0	1381	0	0	1-8	1-8
K	1381	0	1381	0	0	1-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1118	620 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
K	1118	620 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.74 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-F, C-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1477 / 0	-84.3	-84.3	0.40 (1)	4.93
B- C	-1264 / 0	-84.3	-84.3	0.38 (1)	5.24
C- D	-973 / 0	-84.3	-84.3	0.83 (1)	4.74
D- E	-973 / 0	-84.3	-84.3	0.83 (1)	4.74
F- E	-1281 / 0	0.0	0.0	0.38 (1)	5.70
K- A	-1301 / 0	0.0	0.0	0.14 (1)	7.08
K- J	0 / 0	-28.0	-28.0	0.15 (3)	10.00
J- I	0 / 1252	-28.0	-28.0	0.33 (2)	10.00
I- H	0 / 1032	-28.0	-28.0	0.50 (2)	10.00
H- G	0 / 1032	-28.0	-28.0	0.50 (2)	10.00
G- F	0 / 0	-28.0	-28.0	0.38 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- B	-158 / 110	0.06 (1)
B- I	-307 / 0	0.26 (1)
I- C	0 / 419	0.09 (2)
C- G	-87 / 0	0.05 (1)
G- D	-754 / 0	0.95 (1)
G- E	0 / 1431	0.23 (1)
A- J	0 / 1288	0.29 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.83/1.00 (D-E:1) , BC=0.50/1.00 (G-I:2) , WB=0.95/1.00 (D-G:1) , SSI=0.30/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (E) (INPUT = 0.90)
JSI METAL= 0.41 (H) (INPUT = 1.00)

DWG NO. TAM 5015 -18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285956	T45A	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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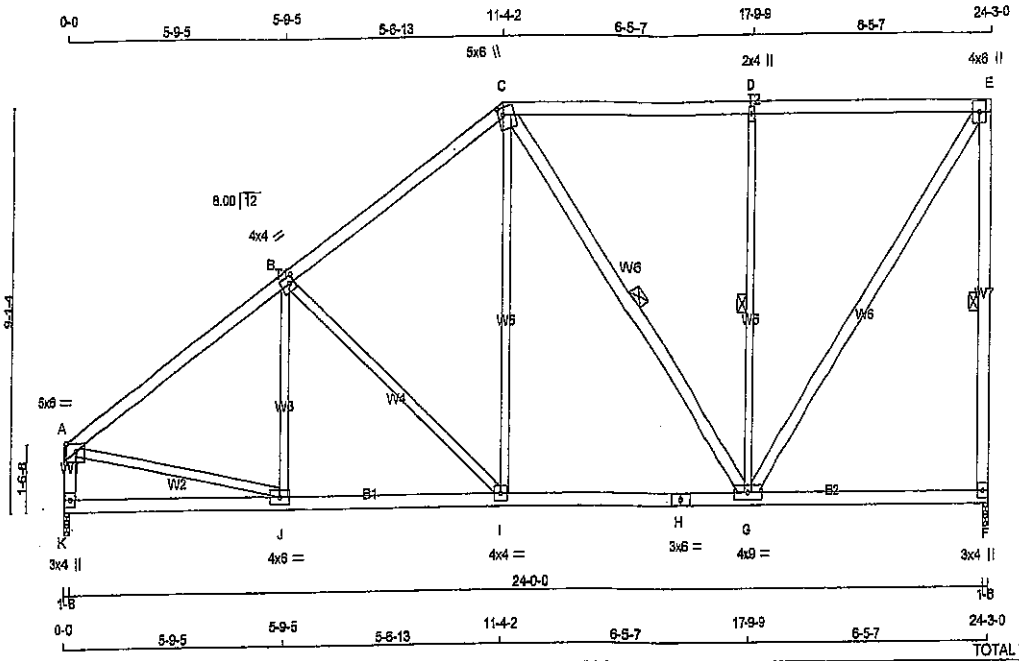

TOTAL WEIGHT = 2 X 121 = 242 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	6.0	Edge	
B	TMWW-t	MT20	4.0	4.0	2.00	1.50
C	TTWW+m	MT20	5.0	8.0	2.00	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVV+p	MT20	4.0	8.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	1361	0	1361	0	0	1-8	1-8
K	1361	0	1361	0	0	1-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1118	620 / 0	255 / 0	0 / 0	0 / 0	242 / 0	0 / 0
K	1118	620 / 0	255 / 0	0 / 0	0 / 0	242 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.70 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, C-G, D-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
FR-TO					
A- B	-1488 / 0	-84.3	-84.3	0.58 (1)	4.70
B- C	-1161 / 0	-84.3	-84.3	0.52 (1)	5.20
C- D	-798 / 0	-84.3	-84.3	0.63 (1)	5.68
D- E	-795 / 0	-84.3	-84.3	0.63 (1)	5.66
F- E	-1288 / 0	0.0	0.0	0.50 (1)	5.69
K- A	-1295 / 0	0.0	0.0	0.14 (1)	7.10
K- J	0 / 0	-28.0	-28.0	0.22 (3)	10.00
J- I	0 / 1262	-28.0	-28.0	0.37 (2)	10.00
I- H	0 / 943	-28.0	-28.0	0.41 (2)	10.00
H- G	0 / 943	-28.0	-28.0	0.41 (2)	10.00
G- F	0 / 0	-28.0	-28.0	0.30 (3)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- B	-89 / 183	0.04 (3)
B- I	-435 / 0	0.51 (1)
I- C	0 / 475	0.11 (2)
C- G	-249 / 0	0.16 (1)
G- D	-675 / 0	0.36 (1)
G- E	0 / 1343	0.22 (1)
A- J	0 / 1290	0.29 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(65 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.63/1.00 (D-E:1) , BC=0.41/1.00 (G-I:2) , WB=0.51/1.00 (B-I:1) , SSI=0.26/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (G) (INPUT = 0.90)
JSI METAL= 0.32 (J) (INPUT = 1.00)

DWG NO. TAM 5984-B
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285956	T46A	3	1	TRUSS DESC.	

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Tamarack Roof Truss, Burlington

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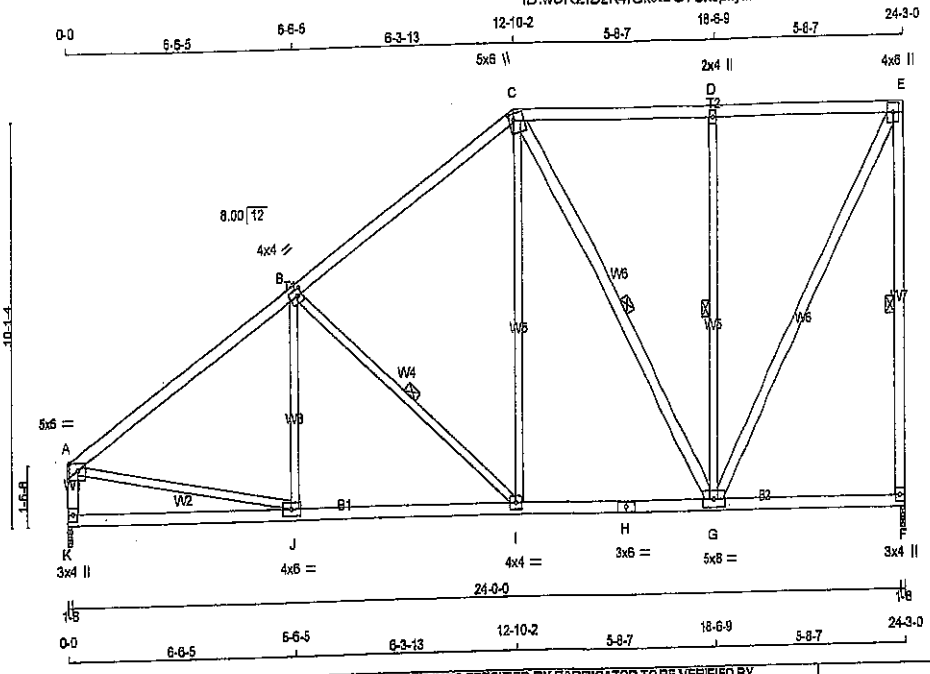

TOTAL WEIGHT = 3 X 127 = 381 lb

[M](F)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	6.0	Edge	
B	TMWW-t	MT20	4.0	4.0	2.00	1.50
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW+p	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	8.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	1361	0	1361	0	0	1-8	1-8
K	1361	0	1361	0	0	1-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1118	620 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
K	1118	620 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, B-I, C-G, D-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1481 / 0	-84.3	-84.3	0.52 (1)	4.77
B- C	-1050 / 0	-84.3	-84.3	0.48 (1)	5.50
C- D	-651 / 0	-84.3	-84.3	0.35 (1)	6.25
D- E	-651 / 0	-84.3	-84.3	0.35 (1)	6.25
F- E	-1298 / 0	0.0	0.0	0.65 (1)	5.67
K- A	-1288 / 0	0.0	0.0	0.13 (1)	7.11
K- J	0 / 0	-28.0	-28.0	0.30 (3)	10.00
J- I	0 / 1262	-28.0	-28.0	0.44 (2)	10.00
I- H	0 / 847	-28.0	-28.0	0.32 (2)	10.00
H- G	0 / 847	-28.0	-28.0	0.32 (2)	10.00
G- F	0 / 0	-28.0	-28.0	0.22 (3)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- B	-29 / 248	0.06 (3)
B- I	-557 / 0	0.27 (1)
I- C	0 / 531	0.12 (2)
C- G	-393 / 0	0.26 (1)
G- D	-595 / 0	0.42 (1)
G- E	0 / 1286	0.21 (1)
A- J	0 / 1284	0.29 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.8	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OSC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(65 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.65/1.00 (E-F:1) , BC=0.44/1.00 (I-J:2) , WB=0.42/1.00 (D-G:1) , SSI=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (J) (INPUT = 0.90)
JSI METAL= 0.32 (J) (INPUT = 1.00)

DWG NO. TAM 5985-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285350	T47	1	1	TRUSS DESC.	

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Tamarack Roof Truss, Burlington

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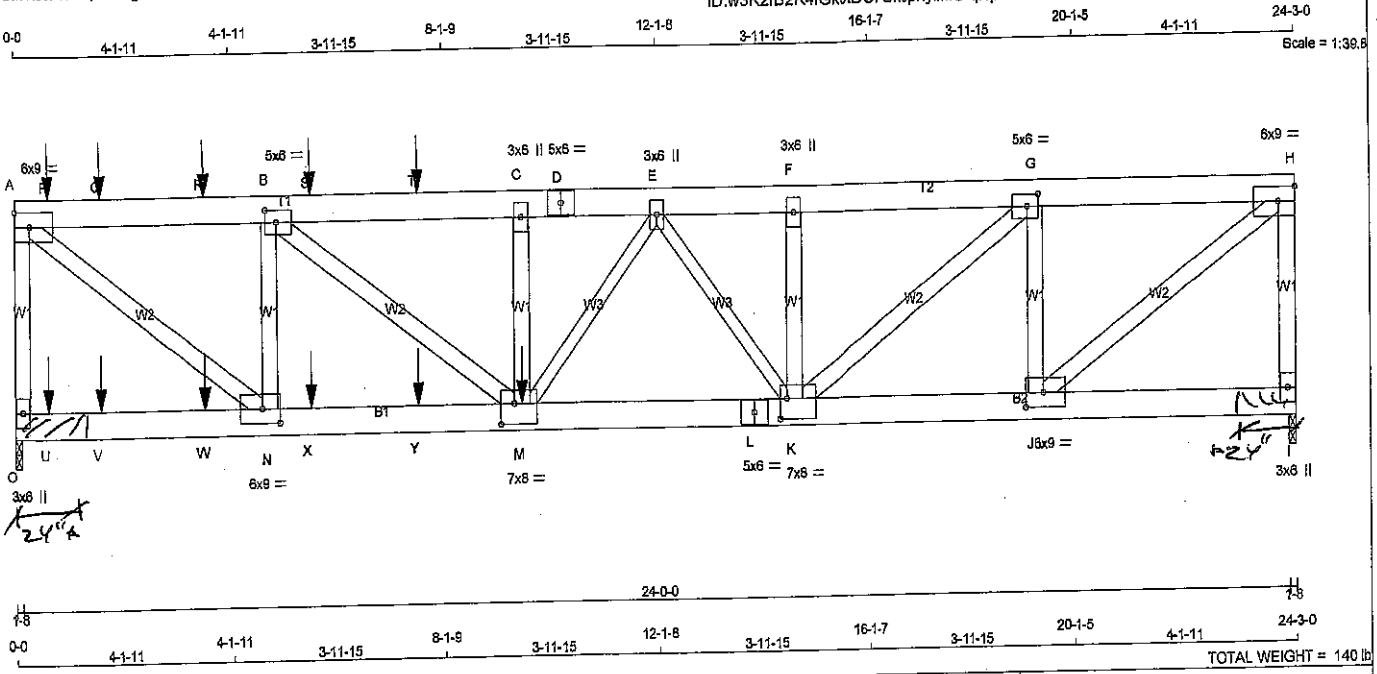

TOTAL WEIGHT = 140 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
O - A	2x4	DRY	No.2	SPF
A - D	2x6	DRY	No.2	SPF
D - H	2x6	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
M - E	2x3	DRY	No.2	SPF
E - K	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-t	MT20	6.0	8.0		Edge
B	TMWW-t	MT20	5.0	6.0	2.50	2.50
C	TMW+w	MT20	3.0	6.0		
D	TS-t	MT20	5.0	6.0		
E	TMWW+t	MT20	3.0	6.0		
F	TMW+w	MT20	3.0	6.0		
G	TMWW-t	MT20	5.0	6.0	2.50	2.50
H	TMWW-t	MT20	6.0	9.0		Edge
I	BMV1+p	MT20	3.0	6.0		
J	BMWW-t	MT20	6.0	9.0	3.00	4.00
K	BMWWW-t	MT20	7.0	8.0	4.25	1.50
L	BS-t	MT20	5.0	6.0		
M	BMWWW-t	MT20	7.0	8.0	4.25	3.00
N	BMWW-t	MT20	6.0	9.0	3.00	4.00
O	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 116.3 lbs FACTORED DOWN AT 7-12, 101.4 lbs FACTORED DOWN AT 1-7-12, 101.4 lbs FACTORED DOWN AT 3-7-12, AND 101.4 lbs FACTORED DOWN AT 5-7-12, AND 101.4 lbs FACTORED DOWN AT 7-7-12 ON TOP CHORD, AND 78.8 lbs FACTORED DOWN AT 7-12, 69.9 lbs FACTORED DOWN AT 1-7-12, 69.9 lbs FACTORED DOWN AT 3-7-12, 69.9 lbs FACTORED DOWN AT 5-7-12, AND 69.9 lbs FACTORED DOWN AT 7-7-12, AND 1628.2 lbs FACTORED DOWN AT 9-7-0 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

SCAB JOINT #O & #I WITH 2 X 6 #2 SPF 24" LONG 1 SIDE(S) USING 2 ROW(S) 3¼" ARDOX SPIRAL/~~COMMON WIRE~~ NAILS AT 3" O/C STAGGERED (5 NAILS/SCAB). REQUIRED FOR 1½" BRG PROVISION.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	2960	0	2960	0	0	1-8	1-8
I	2117	0	2117	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	2427	1352 / 0	550 / 0	0 / 0	0 / 0	525 / 0	0 / 0
I	1731	973 / 0	387 / 0	0 / 0	0 / 0	372 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.47 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
FR-TO					
O-A	-2832 / 0	0.0	0.0	0.65 (1)	5.05
A-P	-3335 / 0	-84.3	-84.3	0.32 (1)	4.34
P-Q	-3335 / 0	-84.3	-84.3	0.32 (1)	4.34
Q-R	-3335 / 0	-84.3	-84.3	0.32 (1)	4.34
R-B	-3335 / 0	-84.3	-84.3	0.32 (1)	4.34
B-S	-5187 / 0	-84.3	-84.3	0.43 (1)	3.47
S-T	-5187 / 0	-84.3	-84.3	0.43 (1)	3.47
T-C	-5187 / 0	-84.3	-84.3	0.43 (1)	3.47
C-D	-5187 / 0	-84.3	-84.3	0.23 (1)	3.69
D-E	-5187 / 0	-84.3	-84.3	0.23 (1)	3.69
E-F	-4165 / 0	-84.3	-84.3	0.15 (1)	4.13
F-G	-4165 / 0	-84.3	-84.3	0.25 (1)	4.04
G-H	-2512 / 0	-84.3	-84.3	0.17 (1)	5.05
I-H	-2053 / 0	0.0	0.0	0.47 (1)	5.85
O-U	0 / 0	-28.0	-28.0	0.15 (2)	10.00
U-V	0 / 0	-28.0	-28.0	0.15 (2)	10.00
V-W	0 / 0	-28.0	-28.0	0.15 (2)	10.00
W-N	0 / 0	-28.0	-28.0	0.15 (2)	10.00
N-X	0 / 3335	-28.0	-28.0	0.54 (1)	10.00
X-Y	0 / 3335	-28.0	-28.0	0.54 (1)	10.00
Y-M	0 / 3335	-28.0	-28.0	0.54 (1)	10.00
M-L	0 / 4744	-28.0	-28.0	0.71 (1)	10.00
L-K	0 / 4744	-28.0	-28.0	0.71 (1)	10.00
K-J	0 / 2512	-28.0	-28.0	0.37 (1)	10.00
J-I	0 / 0	-28.0	-28.0	0.08 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-N	0 / 4167	0.74 (1)
N-B	-2221 / 0	0.38 (1)
B-M	0 / 2341	0.41 (1)
M-C	-315 / 0	0.05 (1)
K-F	-298 / 0	0.05 (1)
K-G	0 / 2088	0.37 (1)
J-G	-1741 / 0	0.30 (1)
J-H	0 / 3140	0.56 (1)
M-E	0 / 775	0.19 (1)
E-K	-1013 / 0	0.33 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
M	9-7-0	-1628	-1628	—	BACK	VERT	TOTAL
P	7-12	-116	-116	—	BACK	VERT	TOTAL
Q	1-7-12	-101	-101	—	BACK	VERT	TOTAL
R	3-7-12	-101	-101	—	BACK	VERT	TOTAL
B	5-7-12	-101	-101	—	BACK	VERT	TOTAL
T	7-7-12	-101	-101	—	BACK	VERT	TOTAL
U	7-12	-45	-79	—	BACK	VERT	TOTAL
V	1-7-12	-40	-70	—	BACK	VERT	TOTAL
W	3-7-12	-40	-70	—	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER S. KATSOULAKOS PROVINCE OF ONTARIO

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.28")

CSI: TC=0.65/1.00 (A-O:1) , BC=0.71/1.00 (K-M:1) , WB=0.74/1.00 (A-N:1) , SSI=0.26/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

USI GRIP= 0.90 (M) (INPUT = 0.90)
USI METAL= 0.76 (L) (INPUT = 1.00)

DWG NO. TAM 8091 -S8
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285350	T47	1	1	TRUSS DESC.	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
X	5-7-12	-40	-70	—	BACK	VERT	TOTAL
Y	7-7-12	-40	-70	—	BACK	VERT	TOTAL

DWG NO. TAM 8091-16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285348	T48	6	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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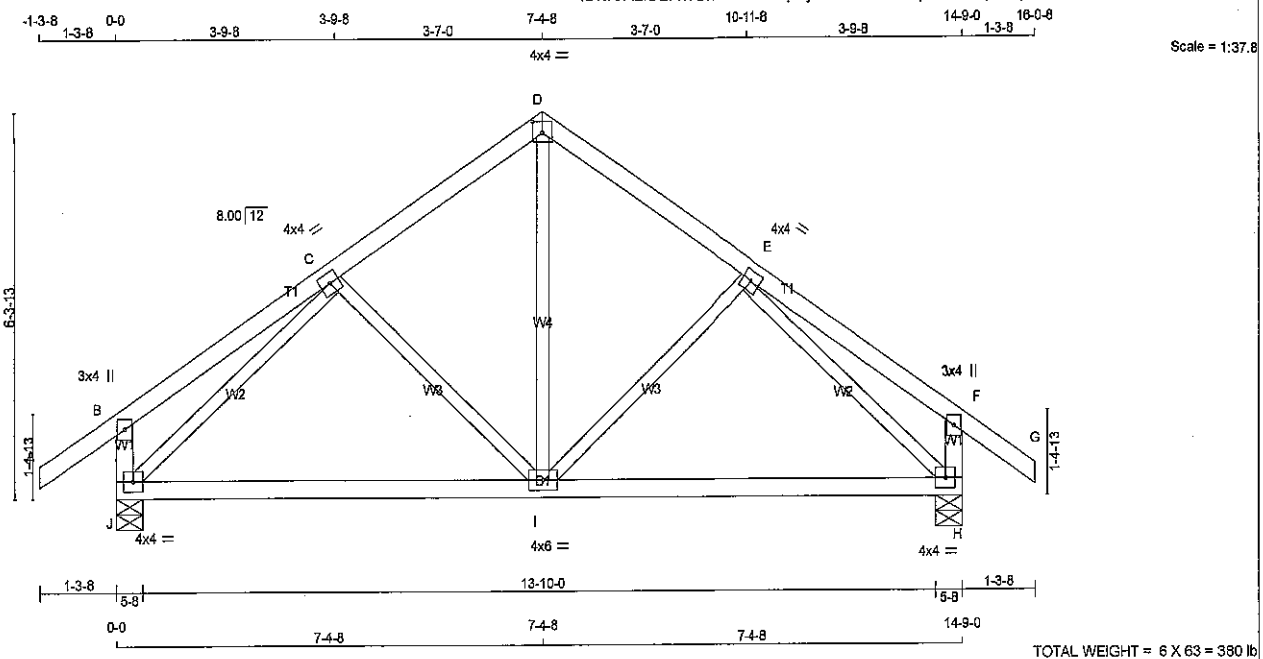

TOTAL WEIGHT = 6 X 63 = 380 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	944	0	944	0	0	5-8	5-8
H	944	0	944	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	758	448 / 0	155 / 0	0 / 0	0 / 0	156 / 0	0 / 0
H	758	448 / 0	155 / 0	0 / 0	0 / 0	156 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.3	-84.3	0.11 (1)		10.00
B- C	0 / 19	-84.3	-84.3	0.18 (1)		10.00
C- D	-668 / 0	-84.3	-84.3	0.14 (1)		6.25
D- E	-668 / 0	-84.3	-84.3	0.14 (1)		6.25
E- F	0 / 19	-84.3	-84.3	0.18 (1)		10.00
F- G	0 / 32	-84.3	-84.3	0.11 (1)		10.00
J- B	-235 / 0	0.0	0.0	0.02 (1)		7.81
H- F	-235 / 0	0.0	0.0	0.02 (1)		7.81
J- I	0 / 655	-28.0	-28.0	0.52 (2)		10.00
I- H	0 / 655	-28.0	-28.0	0.52 (2)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- D	0 / 478	0.11 (1)
I- E	-161 / 43	0.07 (1)
C- I	-161 / 44	0.07 (1)
J- C	-909 / 0	0.38 (1)
E- H	-909 / 0	0.38 (1)

DWG NO. TAM4/1099-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.49")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.49")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.18 (B-C:1) , BC=0.52 (I-J:2) , WB=0.38 (C-J:1) , SSI=0.18 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (D) (INPUT = 0.90)
JSI METAL= 0.32 (C) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285814	T48A	6	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Dec 12 2017 MiTek Industries, Inc. Tue Jan 18 10:36:35 2018 Page 1
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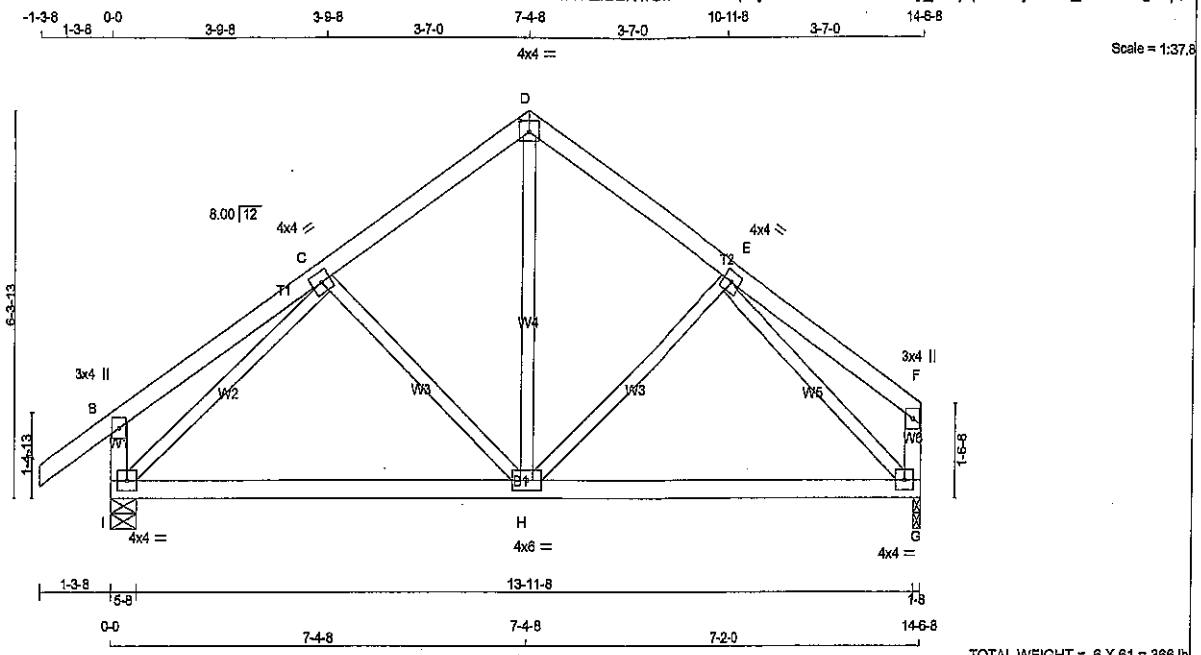


TOTAL WEIGHT = 6 X 61 = 366 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	BMWW1-t	MT20	4.0	4.0		
H	BMWWW-t	MT20	4.0	6.0		
I	BMWW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	932	0	932	0	0	5-8	5-8
G	816	0	816	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	749	442 / 0	153 / 0	0 / 0	0 / 0	154 / 0	0 / 0
G	670	372 / 0	153 / 0	0 / 0	0 / 0	145 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00
B-C	0 / 20	-84.3	-84.3	0.18 (1)	10.00
C-D	-649 / 0	-84.3	-84.3	0.14 (1)	6.25
D-E	-646 / 0	-84.3	-84.3	0.13 (1)	6.25
E-F	0 / 20	-84.3	-84.3	0.17 (1)	10.00
I-B	-235 / 0	0.0	0.0	0.02 (1)	7.81
G-F	-111 / 0	0.0	0.0	0.01 (1)	7.81
I-H	0 / 643	-28.0	-28.0	0.50 (2)	10.00
H-G	0 / 818	-28.0	-28.0	0.50 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H-D	0 / 455	0.10 (1)
H-E	-130 / 53	0.06 (1)
C-H	-164 / 42	0.07 (1)
I-C	-891 / 0	0.37 (1)
E-G	-882 / 0	0.34 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	48.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.48")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.48")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.18/1.00 (B-C:1) , BC=0.50/1.00 (H-I:2) , WB=0.37/1.00 (C-I:1) , SSI=0.17/1.00 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

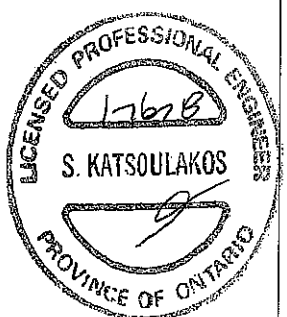
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (E) (INPUT = 0.90)
JSI METAL= 0.32 (C) (INPUT = 1.00)



DWG NO. TAM3983 -18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285745	T48AA	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.210 S May 18 2016 MiTek Industries, Inc. Tue Jun 19 15:28:07 2018 Page 1
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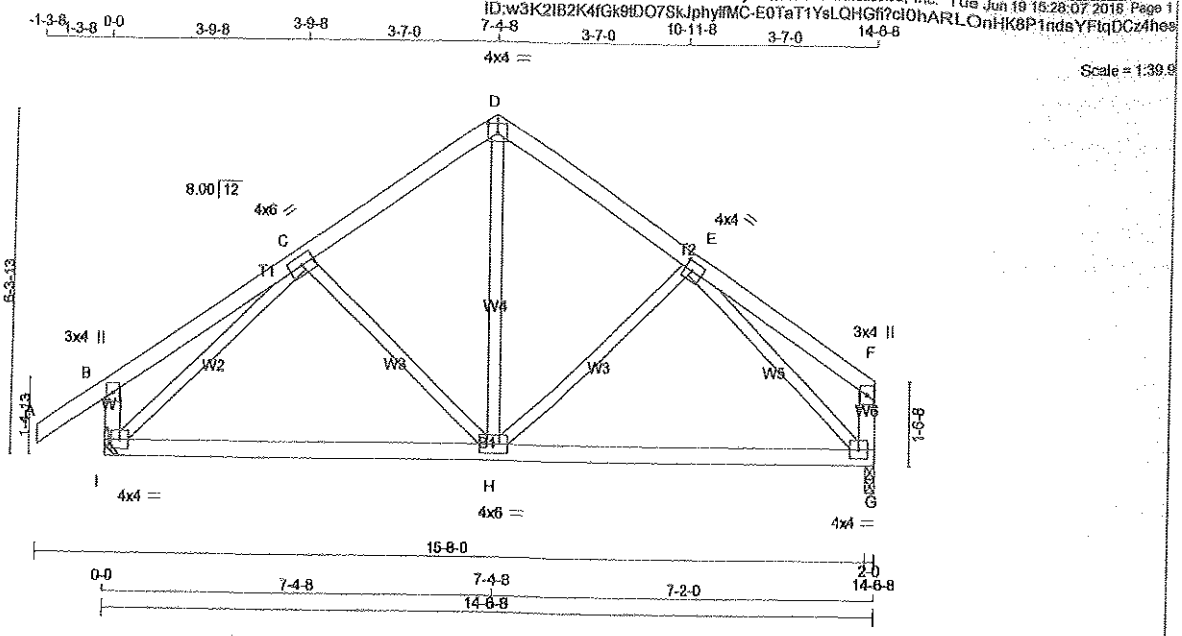

TOTAL WEIGHT = 3 X 61 = 183 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0	2.60	1.75
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BMWWW-t	MT20	4.0	6.0		
I	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	816	0	816	0	0	2-0	2-0
I	932	0	932	0	0	MECHANICAL	

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT I.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	670	372 / 0	153 / 0	0 / 0	0 / 0	145 / 0	0 / 0
I	749	442 / 0	153 / 0	0 / 0	0 / 0	154 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.3	-84.3	0.11 (1)		10.00
B- C	0 / 20	-84.3	-84.3	0.16 (1)		10.00
C- D	-649 / 0	-84.3	-84.3	0.14 (1)		6.25
D- E	-648 / 0	-84.3	-84.3	0.13 (1)		6.25
E- F	0 / 19	-84.3	-84.3	0.17 (1)		10.00
I- B	-236 / 0	0.0	0.0	0.02 (1)		7.81
G- F	-111 / 0	0.0	0.0	0.01 (1)		7.81
I- H	0 / 643	-28.0	-28.0	0.50 (2)		10.00
H- G	0 / 618	-28.0	-28.0	0.50 (2)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- D	0 / 456	0.10 (1)
H- E	-130 / 53	0.09 (1)
C- H	-164 / 41	0.07 (1)
I- C	-891 / 0	0.37 (1)
E- G	-882 / 0	0.34 (1)

LICENSED PROFESSIONAL ENGINEER
06/19/18
H.J.G. ALVES
PROVINCE OF ONTARIO

T1860548

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.48")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.48")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.18/1.00 (B-C:1) , BC=0.50/1.00 (H-I:2) , WB=0.37/1.00 (C-I:1) , SSI=0.17/1.00 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1067	822	2284	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (E) (INPUT = 0.90)
JSI METAL= 0.31 (E) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285348	T49X	6	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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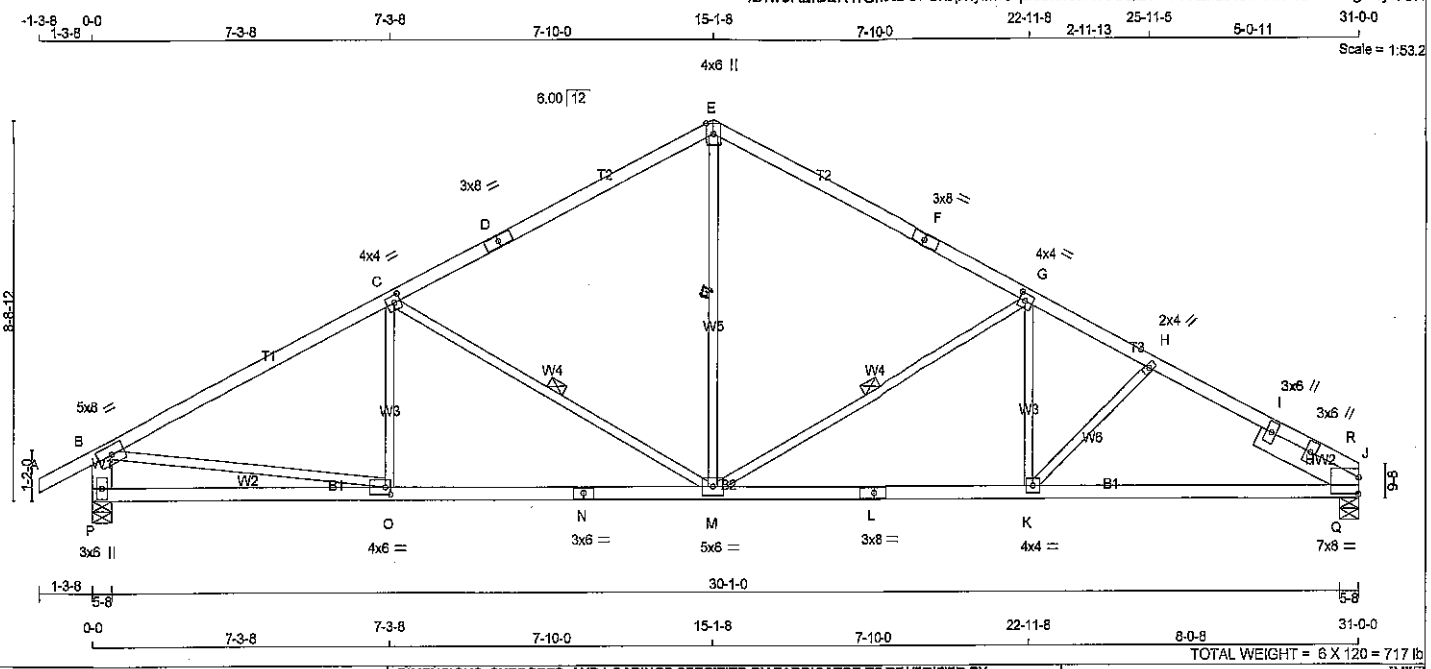


TOTAL WEIGHT = 6 X 120 = 717 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - J	2x4	DRY	No.2	SPF
P - B	2x6	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
REINFORCING MEMBERS				
HW2	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	8.0		
E	TTW+p	MT20	4.0	6.0	Edge	
F	TS-t	MT20	3.0	8.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
H	TMW+w	MT20	2.0	4.0		
J	TMBMR1-I	MT20	7.0	8.0	4.50	
J	RT+t	MT20	3.0	6.0		
J	RT+t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	8.0		
M	BMWWW-t	MT20	5.0	6.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	6.0	2.00	1.50
P	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
P	1854	0	1854	0	0	5-8	5-8
J	1740	0	1740	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1506	863 / 0	326 / 0	0 / 0	0 / 0	318 / 0	0 / 0
J	1429	793 / 0	326 / 0	0 / 0	0 / 0	310 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.41 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

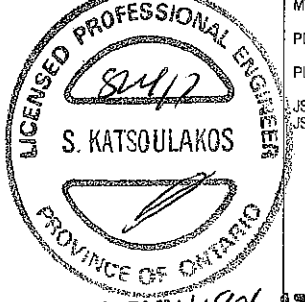
1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-M, G-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 26	-84.3	-84.3	0.11 (1)	10.00
B- C	-2552 / 0	-84.3	-84.3	0.83 (1)	3.41
C- D	-1842 / 0	-84.3	-84.3	0.79 (1)	3.93
D- E	-1842 / 0	-84.3	-84.3	0.79 (1)	3.93
E- F	-1838 / 0	-84.3	-84.3	0.75 (1)	4.03
F- G	-1838 / 0	-84.3	-84.3	0.75 (1)	4.03
G- H	-2627 / 0	-84.3	-84.3	0.60 (1)	3.55
H- I	-2019 / 0	-84.3	-84.3	0.35 (1)	4.43
I- R	-2019 / 0	-84.3	-84.3	0.35 (1)	4.43
R- J	-2363 / 0	-84.3	-84.3	0.13 (1)	4.38
P- B	-1771 / 0	0.0	0.0	0.11 (1)	7.48
P- O	0 / 0	-28.0	-28.0	0.37 (3)	10.00
O- N	0 / 2314	-28.0	-28.0	0.64 (2)	10.00
N- M	0 / 2314	-28.0	-28.0	0.64 (2)	10.00
M- L	0 / 2435	-28.0	-28.0	0.63 (2)	10.00
L- K	0 / 2435	-28.0	-28.0	0.63 (2)	10.00
K- Q	0 / 2370	-28.0	-28.0	0.58 (2)	10.00
Q- J	0 / 1281	-28.0	-28.0	0.48 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- C	-43 / 276	0.06 (3)
C- M	-803 / 0	0.46 (1)
M- E	0 / 1096	0.25 (1)
M- G	-943 / 0	0.54 (1)
K- G	0 / 331	0.07 (3)
B- O	0 / 2330	0.52 (1)
K- H	0 / 92	0.02 (1)
Q- R	0 / 1434	0.00 (1)
Q- I	-1506 / 0	0.12 (1)



DWG NO. TAM4L901-17
STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.30")

CSI: TC=0.83 (B-C:1) , BC=0.64 (M-O:2) , WB=0.54 (G-M:1) , SSI=0.35 (J-Q:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (O) (INPUT = 0.90)
JSI METAL= 0.80 (L) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285348	T50	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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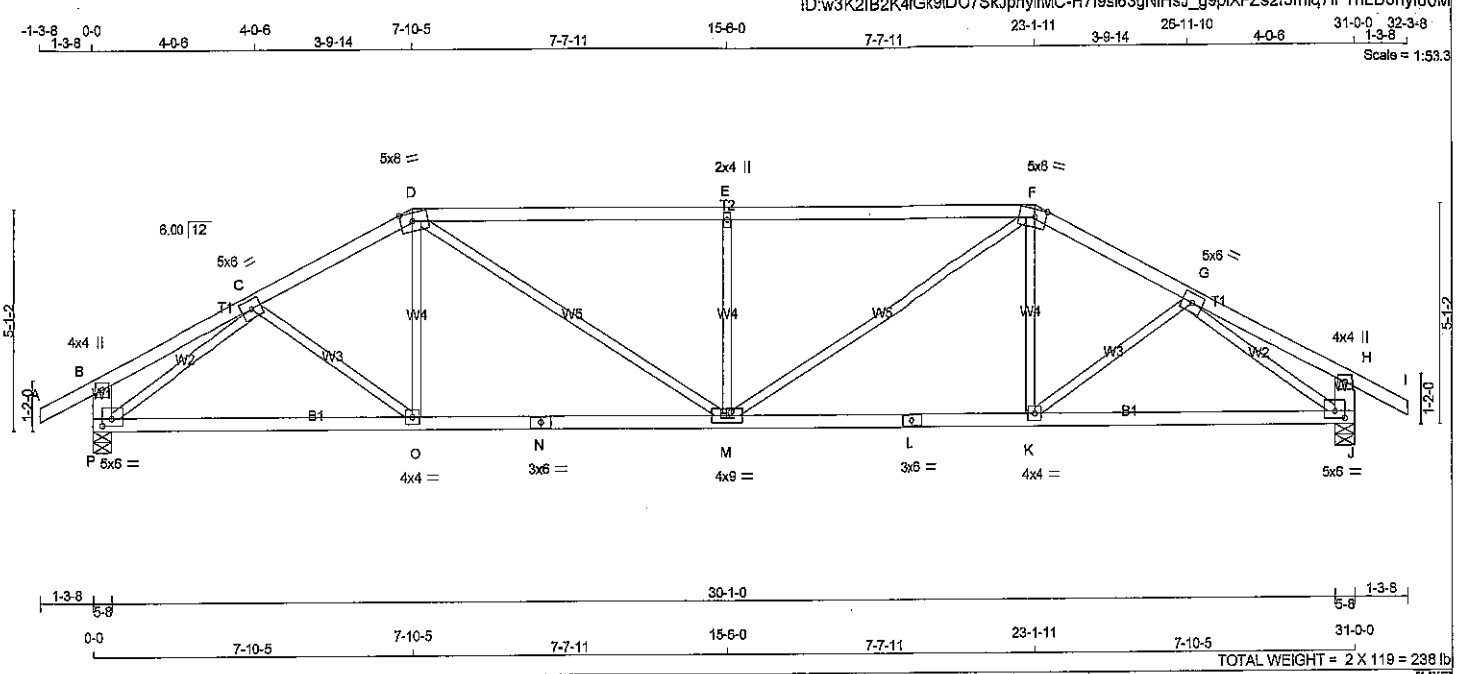

TOTAL WEIGHT = 2 X 119 = 238 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
P - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW-m	MT20	5.0	6.0	2.25	3.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	3.50
G	TMWW-t	MT20	5.0	6.0		
H	TMV+p	MT20	4.0	4.0		
J	BMVW1-t	MT20	5.0	6.0	2.00	2.75
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	4.0	9.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMVW1-t	MT20	5.0	6.0	2.00	2.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	1854	0	1854	0	0	5-8	5-8
J	1854	0	1854	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1506	863 / 0	326 / 0	0 / 0	0 / 0	318 / 0	0 / 0
J	1506	863 / 0	326 / 0	0 / 0	0 / 0	318 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.08 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 26	-84.3	-84.3	1	0.11 (1)	10.00	C- O	0 / 186	0.04 (3)
B- C	0 / 14	-84.3	-84.3	1	0.17 (1)	10.00	O- D	0 / 339	0.08 (2)
C- D	-2435 / 0	-84.3	-84.3	1	0.21 (1)	4.25	D- M	0 / 923	0.21 (1)
D- E	-2949 / 0	-84.3	-84.3	1	0.68 (1)	3.08	M- E	-792 / 0	0.30 (1)
E- F	-2949 / 0	-84.3	-84.3	1	0.68 (1)	3.08	M- F	0 / 923	0.21 (1)
F- G	-2435 / 0	-84.3	-84.3	1	0.21 (1)	4.25	K- F	0 / 339	0.08 (2)
G- H	0 / 14	-84.3	-84.3	1	0.17 (1)	10.00	K- G	0 / 186	0.04 (3)
H- I	0 / 26	-84.3	-84.3	1	0.11 (1)	10.00	P- C	-2601 / 0	0.94 (1)
P- B	-248 / 0	0.0	0.0	1	0.02 (1)	7.81	G- J	-2601 / 0	0.94 (1)
J- H	-248 / 0	0.0	0.0	1	0.02 (1)	7.81			
P- O	0 / 2113	-28.0	-28.0	2	0.67 (2)	10.00			
O- N	0 / 2167	-28.0	-28.0	2	0.69 (2)	10.00			
N- M	0 / 2167	-28.0	-28.0	2	0.69 (2)	10.00			
M- L	0 / 2167	-28.0	-28.0	2	0.69 (2)	10.00			
L- K	0 / 2167	-28.0	-28.0	2	0.69 (2)	10.00			
K- J	0 / 2113	-28.0	-28.0	2	0.67 (2)	10.00			

DWG NO. TAM 4/902-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.30")

CSI: TC=0.68 (D-E:1) , BC=0.69 (K-M:2) , WB=0.94 (G-J:1) , SSI=0.31 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1567	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (J) (INPUT = 0.90)
JSI METAL= 0.65 (N) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285348	T51	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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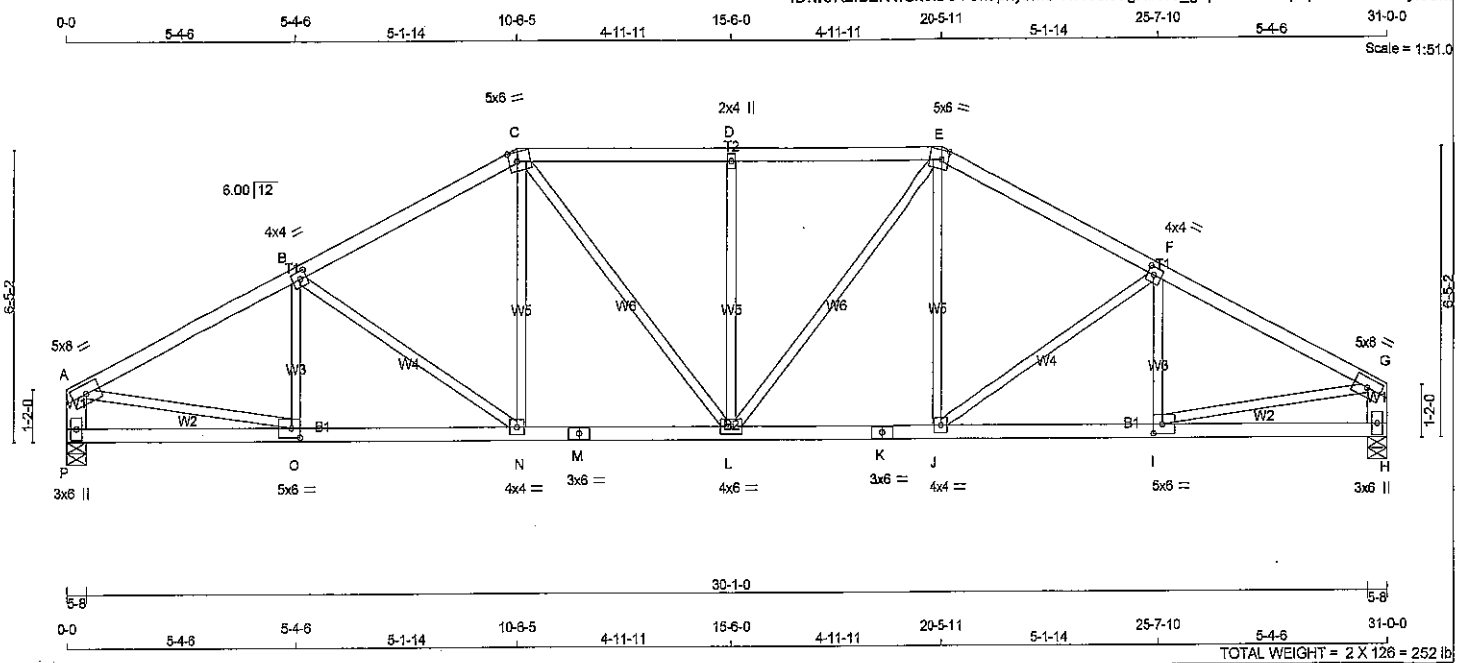


[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
P - A	2x6	DRY	No.2	SPF
H - G	2x6	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	8.0		
B	TMWW-t	MT20	4.0	4.0	2.00	1.75
C	TTWW-m	MT20	5.0	6.0	2.50	2.25
D	TMW+w	MT20	2.0	4.0		
E	TTWW-m	MT20	5.0	6.0	2.50	2.25
F	TMWW-t	MT20	4.0	4.0	2.00	1.75
G	TMVW-t	MT20	5.0	8.0		
H	BMV1+p	MT20	3.0	6.0		
I	BMWW-t	MT20	5.0	6.0	2.50	2.50
J	BMWW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	4.0	6.0		
M	BS-t	MT20	3.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMWW-t	MT20	5.0	6.0	2.50	2.50
P	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
P	1740	0	1740	0	0	5-8	5-8
H	1740	0	1740	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1429	793 / 0	326 / 0	0 / 0	0 / 0	310 / 0	0 / 0
H	1429	793 / 0	326 / 0	0 / 0	0 / 0	310 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, H

BRACING

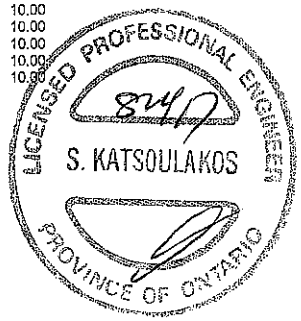
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.01 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)		LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO			FR-TO		
A- B	-2517 / 0	-84.3	-84.3	0.39 (1)	4.01	O- B	-204 / 111	0.05 (1)
B- C	-2237 / 0	-84.3	-84.3	0.36 (1)	4.24	B- N	-353 / 0	0.25 (1)
C- D	-2244 / 0	-84.3	-84.3	0.33 (1)	4.27	N- C	0 / 380	0.09 (2)
D- E	-2244 / 0	-84.3	-84.3	0.33 (1)	4.27	C- L	0 / 412	0.09 (1)
E- F	-2237 / 0	-84.3	-84.3	0.36 (1)	4.24	L- D	-510 / 0	0.34 (1)
F- G	-2517 / 0	-84.3	-84.3	0.39 (1)	4.01	L- E	0 / 412	0.09 (1)
P- A	-1675 / 0	0.0	0.0	0.11 (1)	7.63	J- E	0 / 380	0.09 (2)
H- G	-1675 / 0	0.0	0.0	0.11 (1)	7.63	J- F	-353 / 0	0.25 (1)
						I- F	-204 / 111	0.05 (1)
P- O	0 / 0	-28.0	-28.0	0.18 (3)	10.00	A- O	0 / 2300	0.52 (1)
O- N	0 / 2271	-28.0	-28.0	0.44 (1)	10.00	I- G	0 / 2300	0.52 (1)
N- M	0 / 1984	-28.0	-28.0	0.40 (1)	10.00			
M- L	0 / 1984	-28.0	-28.0	0.40 (1)	10.00			
L- K	0 / 1984	-28.0	-28.0	0.40 (1)	10.00			
K- J	0 / 1984	-28.0	-28.0	0.40 (1)	10.00			
J- I	0 / 2271	-28.0	-28.0	0.44 (1)	10.00			
I- H	0 / 0	-28.0	-28.0	0.18 (3)	10.00			



DWG NO. TAM 4/903-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.39 (A-B:1) , BC=0.44 (N-O:1) , WB=0.52 (A-O:1) , SSI=0.20 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (I) (INPUT = 0.90)
JSI METAL= 0.57 (O) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285348	T52	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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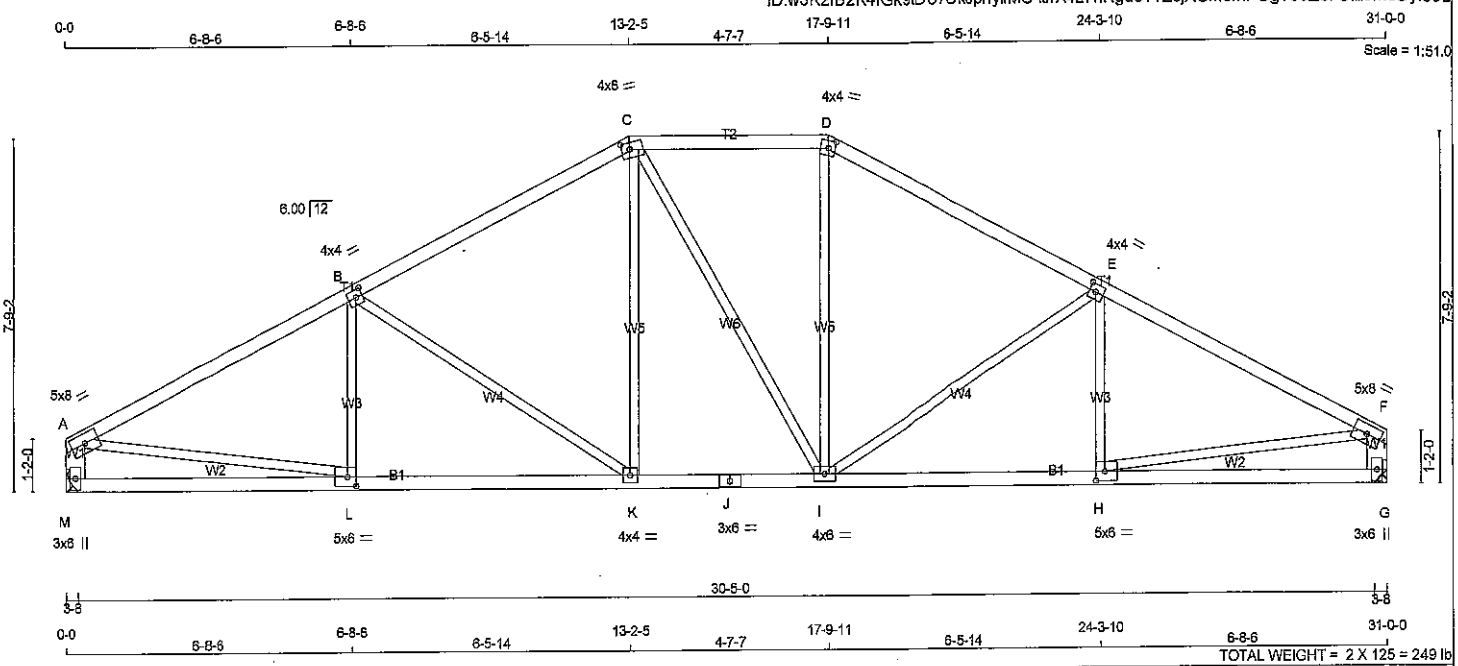


TOTAL WEIGHT = 2 X 125 = 249 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
M - A	2x6	DRY	No.2	SPF
G - F	2x6	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0		
B	TMWW-t	MT20	4.0	4.0	2.00	1.75
C	TTWW-m	MT20	4.0	6.0	1.75	2.25
D	TTW-m	MT20	4.0	4.0	2.00	1.75
E	TMWW-t	MT20	4.0	4.0	2.00	1.75
F	TMVW-t	MT20	5.0	6.0		
G	BMV1+p	MT20	3.0	6.0		
H	BMWW-t	MT20	5.0	6.0	2.50	2.50
I	BMWWW-t	MT20	4.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.50
M	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1740	0	1740	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	
G	1740	0	1740	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1429	793 / 0	326 / 0	0 / 0	0 / 0	310 / 0	0 / 0
G	1429	793 / 0	326 / 0	0 / 0	0 / 0	310 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.71 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

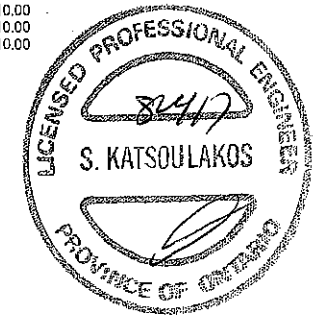
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-2545 / 0	-84.3	-84.3	0.64 (1)		3.71
B- C	-2010 / 0	-84.3	-84.3	0.56 (1)		4.17
C- D	-1777 / 0	-84.3	-84.3	0.27 (1)		4.75
D- E	-2010 / 0	-84.3	-84.3	0.56 (1)		4.17
E- F	-2544 / 0	-84.3	-84.3	0.64 (1)		3.71
M- A	-1663 / 0	0.0	0.0	0.11 (1)		7.65
G- F	-1663 / 0	0.0	0.0	0.11 (1)		7.65
M- L	0 / 0	-28.0	-28.0	0.31 (3)		10.00
L- K	0 / 2302	-28.0	-28.0	0.57 (2)		10.00
K- J	0 / 1776	-28.0	-28.0	0.38 (1)		10.00
J- I	0 / 1776	-28.0	-28.0	0.38 (1)		10.00
I- H	0 / 2302	-28.0	-28.0	0.57 (2)		10.00
H- G	0 / 0	-28.0	-28.0	0.31 (3)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- B	-84 / 230	0.05 (3)
B- K	-632 / 0	0.80 (1)
K- C	0 / 492	0.11 (2)
C- I	0 / 2	0.00 (2)
I- D	0 / 494	0.11 (2)
I- E	-630 / 0	0.80 (1)
H- E	-85 / 229	0.05 (3)
A- L	0 / 2321	0.52 (1)
H- F	0 / 2320	0.52 (1)



DWG NO. TAM 4904-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.64 (A-B:1) , BC=0.57 (K-L:2) , WB=0.80 (B-K:1) , SSI=0.24 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (L) (INPUT = 0.90)
JSI METAL= 0.58 (L) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285348	T54S	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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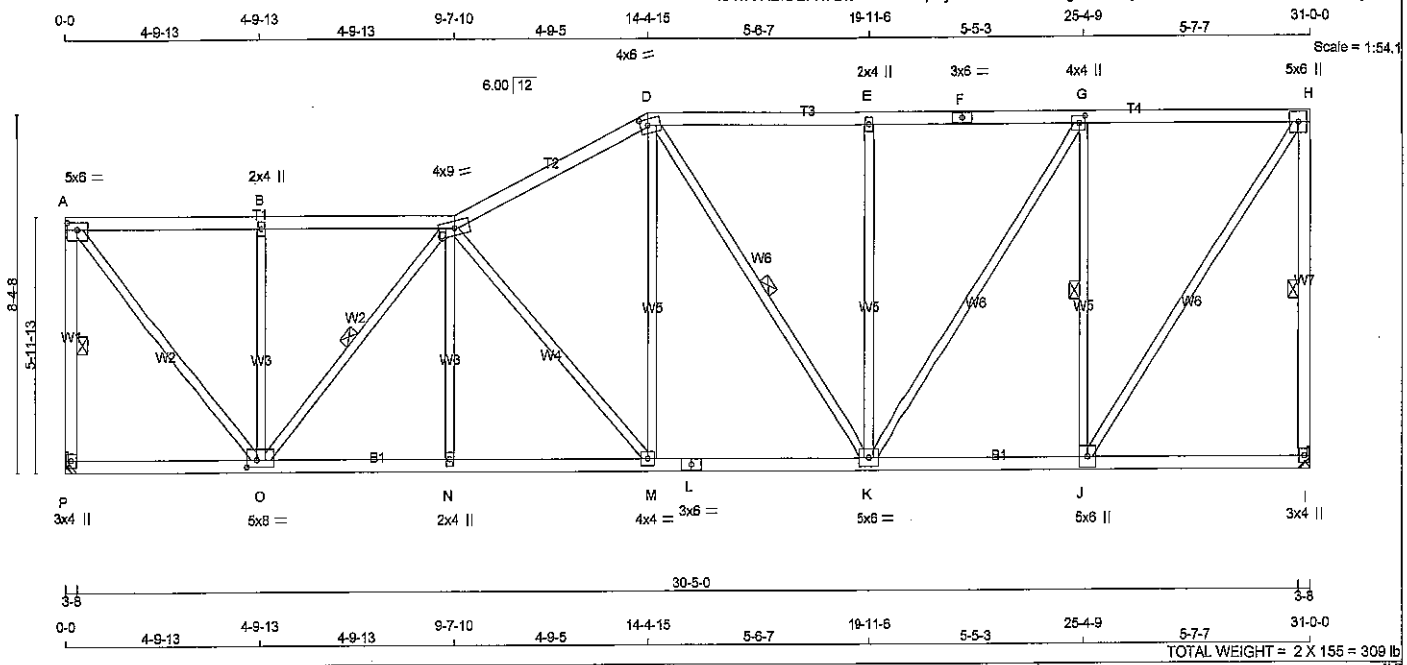


[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
P - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
P - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0	2.00	3.00
B	TMW+w	MT20	2.0	4.0		
C	TTWWW-m	MT20	4.0	9.0		
D	TTWW-m	MT20	4.0	6.0	1.75	2.25
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW+t	MT20	4.0	4.0	2.00	1.75
H	TMVW+p	MT20	5.0	6.0		
I	BMV1+p	MT20	3.0	4.0		
J	BMWW+t	MT20	5.0	6.0		
K	BMWWW-t	MT20	5.0	6.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMW+w	MT20	2.0	4.0		
O	BMWWW-t	MT20	5.0	8.0	2.00	3.00
P	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
P	1740	0	1740	0	0	HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-8	
I	1740	0	1740	0	0	HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1429	793 / 0	325 / 0	0 / 0	0 / 0	310 / 0	0 / 0
I	1429	793 / 0	325 / 0	0 / 0	0 / 0	310 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.47 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

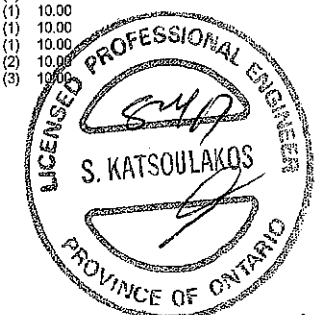
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF A-P, H-I, C-O, D-K, G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS						WEBS		
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- A	-1684 / 0	0.0	0.0	0.29 (1)	5.12	A- O	0 / 2007	0.45 (1)
A- B	-1303 / 0	-84.3	-84.3	0.36 (1)	5.21	O- B	-499 / 0	0.28 (1)
B- C	-1303 / 0	-84.3	-84.3	0.35 (1)	5.21	O- C	-1144 / 0	0.41 (1)
C- D	-1860 / 0	-84.3	-84.3	0.41 (1)	4.47	N- C	0 / 219	0.05 (3)
D- E	-1567 / 0	-84.3	-84.3	0.40 (1)	4.81	C- M	-573 / 0	0.64 (1)
E- F	-1568 / 0	-84.3	-84.3	0.41 (1)	4.79	M- D	0 / 590	0.13 (1)
F- G	-1568 / 0	-84.3	-84.3	0.41 (1)	4.79	J- H	0 / 1808	0.41 (1)
G- H	-1032 / 0	-84.3	-84.3	0.38 (1)	5.65	D- K	-184 / 0	0.12 (1)
I- H	-1677 / 0	0.0	0.0	0.53 (1)	5.13	J- G	-1308 / 0	0.58 (1)
						K- E	-501 / 0	0.70 (1)
P- O	0 / 0	-28.0	-28.0	0.16 (3)	10.00	K- G	0 / 959	0.22 (1)
O- N	0 / 2039	-28.0	-28.0	0.41 (1)	10.00			
N- M	0 / 2037	-28.0	-28.0	0.42 (1)	10.00			
M- L	0 / 1671	-28.0	-28.0	0.36 (1)	10.00			
L- K	0 / 1671	-28.0	-28.0	0.36 (1)	10.00			
K- J	0 / 1032	-28.0	-28.0	0.33 (2)	10.00			
J- I	0 / 0	-28.0	-28.0	0.21 (3)	10.00			



DWG NO. TAM 4/905-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.53 (H-I:1) , BC=0.42 (M-N:1) , WB=0.70 (E-K:1) , SSI=0.22 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (G) (INPUT = 0.90)
JSI METAL= 0.67 (C) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285348	T55S	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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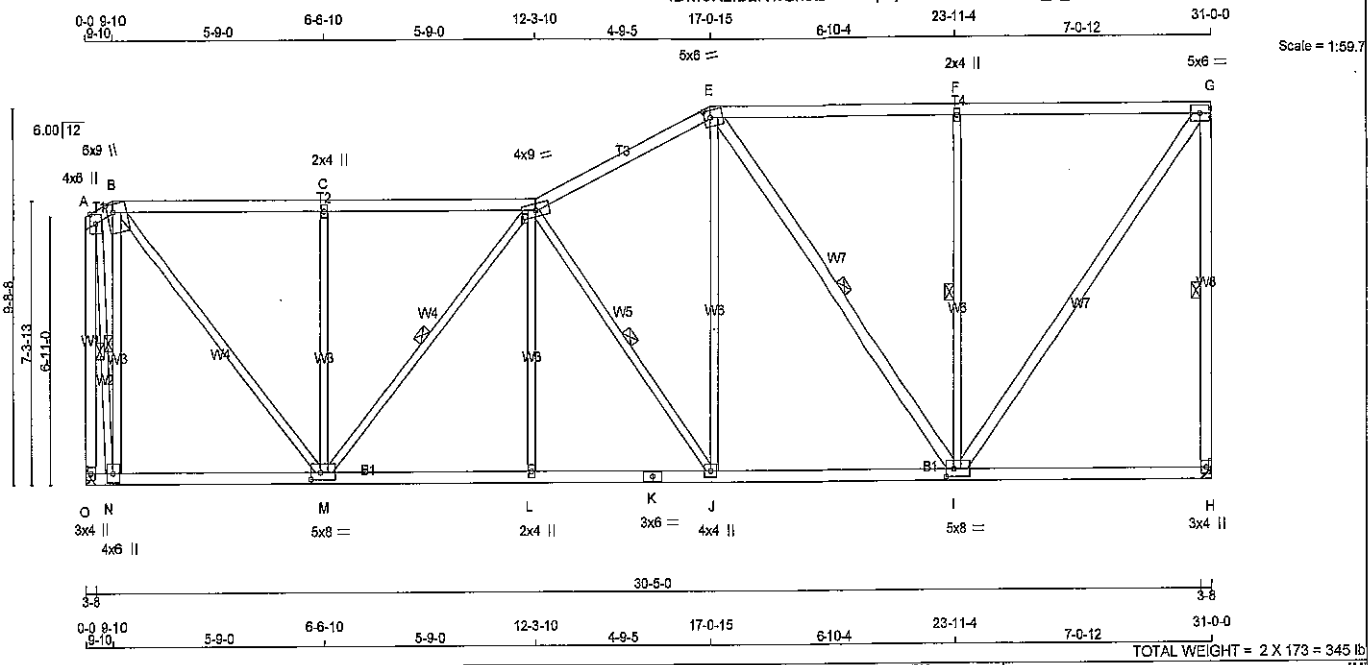


Scale = 1:59.7

TOTAL WEIGHT = 2 X 173 = 345 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
O - A	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
B	TTWW+m	MT20	6.0	9.0	2.50	2.00
C	TMW+w	MT20	2.0	4.0		
D	TTWWW-m	MT20	4.0	9.0		
E	TTWW-m	MT20	5.0	6.0	2.25	2.00
F	TMW+w	MT20	2.0	4.0		
G	TMVW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWWW-t	MT20	5.0	8.0	2.50	2.50
J	BMWW+t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMW+w	MT20	2.0	4.0		
M	BMWWW-t	MT20	5.0	8.0	2.25	3.00
N	BMWW+t	MT20	4.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	1740	0	1740	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	
O	1740	0	1740	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1429	793 / 0	325 / 0	0 / 0	0 / 0	310 / 0	0 / 0
O	1429	793 / 0	325 / 0	0 / 0	0 / 0	310 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.74 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-H, B-N, D-M, D-J, E-I, F-I, A-O.

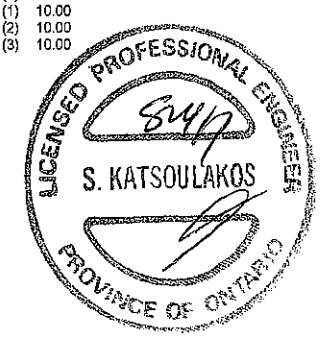
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-189 / 0	-84.3	-84.3	0.01 (1)	6.25
B- C	-1341 / 0	-84.3	-84.3	0.53 (1)	4.88
C- D	-1341 / 0	-84.3	-84.3	0.53 (1)	4.88
D- E	-1586 / 0	-84.3	-84.3	0.39 (1)	4.79
E- F	-1070 / 0	-84.3	-84.3	0.78 (1)	4.74
F- G	-1070 / 0	-84.3	-84.3	0.78 (1)	4.74
H- G	-1662 / 0	0.0	0.0	0.75 (1)	5.15
O- A	-1694 / 0	0.0	0.0	0.38 (1)	5.11
O- N	0 / 0	-28.0	-28.0	0.09 (3)	10.00
N- M	0 / 189	-28.0	-28.0	0.24 (2)	10.00
M- L	0 / 1847	-28.0	-28.0	0.44 (2)	10.00
L- K	0 / 1845	-28.0	-28.0	0.40 (1)	10.00
K- J	0 / 1845	-28.0	-28.0	0.40 (1)	10.00
J- I	0 / 1428	-28.0	-28.0	0.52 (2)	10.00
I- H	0 / 0	-28.0	-28.0	0.35 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- B	-1540 / 0	0.52 (1)
B- M	0 / 1618	0.41 (1)
M- C	-597 / 0	0.57 (1)
M- D	-797 / 0	0.45 (1)
I- D	0 / 242	0.05 (3)
D- J	-745 / 0	0.37 (1)
J- E	0 / 774	0.17 (1)
E- I	-604 / 0	0.46 (1)
I- F	-729 / 0	0.46 (1)
I- G	0 / 1793	0.29 (1)
A- N	0 / 1584	0.36 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.78 (F-G:1) , BC=0.52 (I-J:2) , WB=0.57 (C-M:1) , SSI=0.29 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (N) (INPUT = 0.90)
JSI METAL= 0.54 (D) (INPUT = 1.00)

DWG NO. TAM41906-17
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285348	T56S	2	1	TRUSS DESC.	

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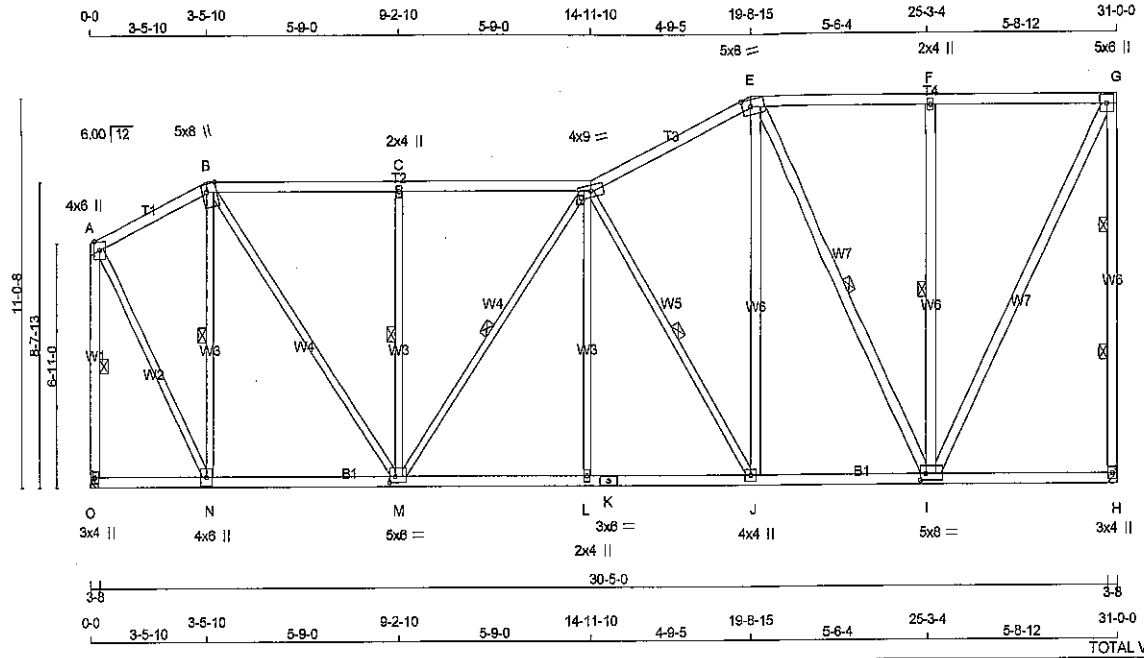

TOTAL WEIGHT = 2 X 191 = 382 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
O - A	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
J - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
B	TTWW+m	MT20	5.0	8.0	Edge	3.50
C	TMW+w	MT20	2.0	4.0		
D	TTWWW-m	MT20	4.0	9.0		
E	TTWW-m	MT20	5.0	8.0	2.25	3.00
F	TMW+w	MT20	2.0	4.0		
G	TMVW+p	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWWW-t	MT20	5.0	8.0	2.25	2.00
J	BMWW+t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMW+w	MT20	2.0	4.0		
M	BMWWW-t	MT20	5.0	6.0	2.00	2.00
N	BMWW+t	MT20	4.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1740	0	1740	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	
O	1740	0	1740	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1429	793 / 0	325 / 0	0 / 0	0 / 0	310 / 0	0 / 0
O	1429	793 / 0	325 / 0	0 / 0	0 / 0	310 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.80 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2 LATERAL BRACE(S) AT 1/3 LENGTH OF G-H.
1 LATERAL BRACE(S) AT 1/2 LENGTH OF B-N, C-M, D-M, D-J, E-I, F-I, A-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	-743 / 0	-84.3	-84.3	0.16 (1)	6.25
B-C	-1396 / 0	-84.3	-84.3	0.53 (1)	4.80
C-D	-1396 / 0	-84.3	-84.3	0.53 (1)	4.80
D-E	-1291 / 0	-84.3	-84.3	0.38 (1)	5.21
E-F	-784 / 0	-84.3	-84.3	0.47 (1)	6.07
F-G	-784 / 0	-84.3	-84.3	0.47 (1)	6.07
H-G	-1676 / 0	0.0	0.0	0.46 (1)	5.13
O-A	-1708 / 0	0.0	0.0	0.38 (1)	5.09
O-N	0 / 0	-28.0	-28.0	0.15 (2)	10.00
N-M	0 / 650	-28.0	-28.0	0.26 (2)	10.00
M-L	0 / 1621	-28.0	-28.0	0.37 (2)	10.00
L-K	0 / 1619	-28.0	-28.0	0.34 (1)	10.00
K-J	0 / 1619	-28.0	-28.0	0.34 (1)	10.00
J-I	0 / 1167	-28.0	-28.0	0.36 (2)	10.00
I-H	0 / 0	-28.0	-28.0	0.22 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N-B	-1082 / 0	0.52 (1)
B-M	0 / 1316	0.30 (1)
M-C	-597 / 0	0.29 (1)
M-D	-395 / 0	0.29 (1)
L-D	0 / 258	0.05 (3)
D-J	-913 / 0	0.61 (1)
J-E	0 / 928	0.15 (1)
E-I	-827 / 0	0.69 (1)
I-F	-588 / 0	0.37 (1)
I-G	0 / 1690	0.27 (1)
A-N	0 / 1396	0.31 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.53 (B-C:1) , BC=0.37 (L-M:2) , WB=0.69 (E-I:1) , SSI=0.23 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (J) (INPUT = 0.90)
JSI METAL= 0.44 (K) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 41 90717
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285348	T57	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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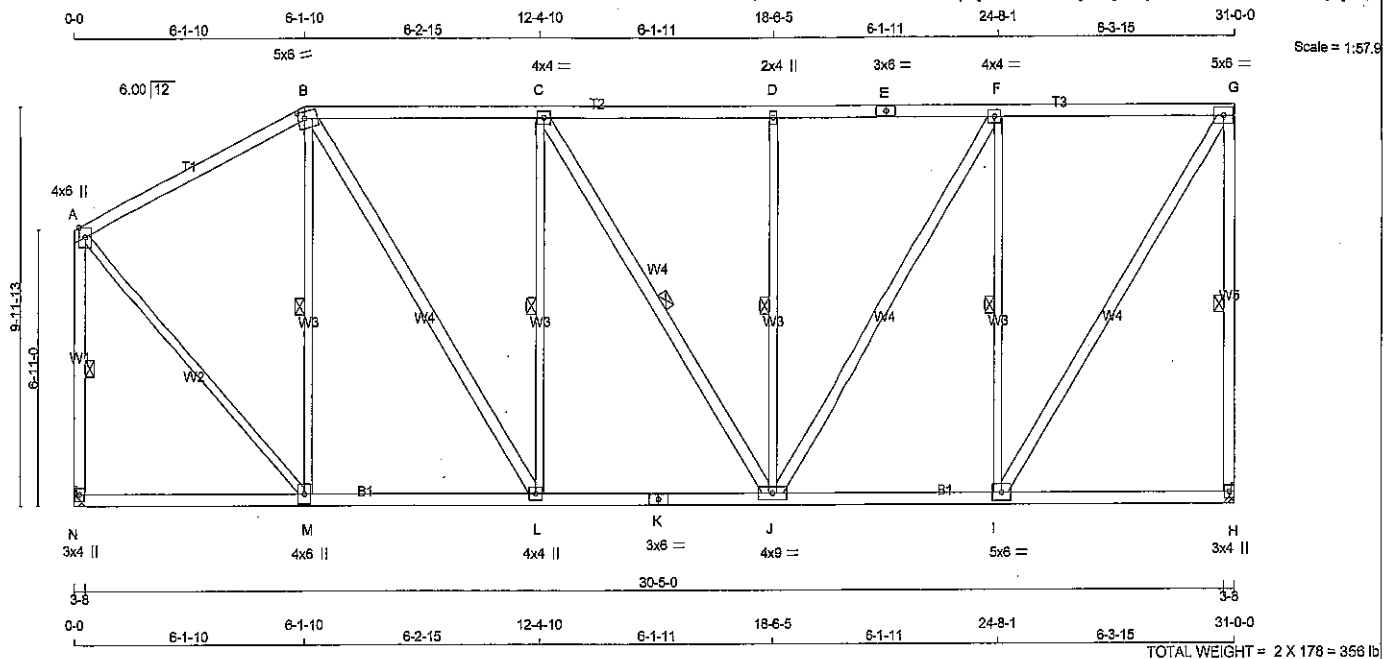


TOTAL WEIGHT = 2 X 178 = 356 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
N - A	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
B - L	2x4	DRY	No.2	SPF
J - F	2x4	DRY	No.2	SPF
C - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
B	TTWW-m	MT20	5.0	6.0	2.00	2.00
C	TMWW-t	MT20	4.0	4.0		
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	5.0	6.0		
J	BMWWW-t	MT20	4.0	9.0		
K	BS-t	MT20	3.0	6.0		
L	BMWW+t	MT20	4.0	4.0		
M	BMWW+t	MT20	4.0	6.0		
N	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1740	0	1740	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	
N	1740	0	1740	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1429	793 / 0	325 / 0	0 / 0	0 / 0	310 / 0	0 / 0
N	1429	793 / 0	325 / 0	0 / 0	0 / 0	310 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.80 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

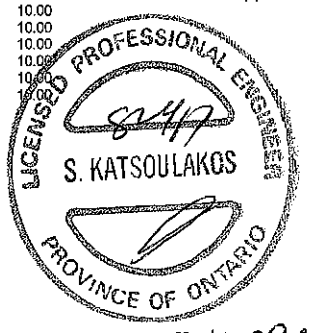
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-H, B-M, A-N, F-I, C-L, C-J, D-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF)	CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1012 / 0	-84.3	-84.3	0.61 (1)		5.27
B- C	-1379 / 0	-84.3	-84.3	0.56 (1)		4.80
C- D	-1368 / 0	-84.3	-84.3	0.56 (1)		4.82
D- E	-1368 / 0	-84.3	-84.3	0.56 (1)		4.80
E- F	-1368 / 0	-84.3	-84.3	0.56 (1)		4.80
F- G	-952 / 0	-84.3	-84.3	0.53 (1)		5.55
H- G	-1669 / 0	0.0	0.0	0.81 (1)		5.14
N- A	-1671 / 0	0.0	0.0	0.37 (1)		5.14
N- M	0 / 0	-28.0	-28.0	0.27 (3)		10.00
M- L	0 / 897	-28.0	-28.0	0.38 (2)		10.00
L- K	0 / 1379	-28.0	-28.0	0.36 (2)		10.00
K- J	0 / 1379	-28.0	-28.0	0.36 (2)		10.00
J- I	0 / 952	-28.0	-28.0	0.38 (2)		10.00
I- H	0 / 0	-28.0	-28.0	0.27 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- B	-762 / 0	0.52 (1)
A- M	0 / 1318	0.30 (1)
I- G	0 / 1741	0.28 (1)
B- L	0 / 889	0.14 (1)
I- F	-1258 / 0	0.85 (1)
L- C	-579 / 0	0.39 (1)
J- F	0 / 776	0.12 (1)
C- J	-20 / 0	0.02 (1)
J- D	-475 / 0	0.32 (1)



DWG NO. TAM 41908-17
STRUCTURAL
COMPONENT ONLY

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.81 (G-H:1) , BC=0.38 (I-J:2) , WB=0.85 (F-I:1) , SSI=0.25 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (B) (INPUT = 0.90)
JSI METAL= 0.47 (K) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285348	T58	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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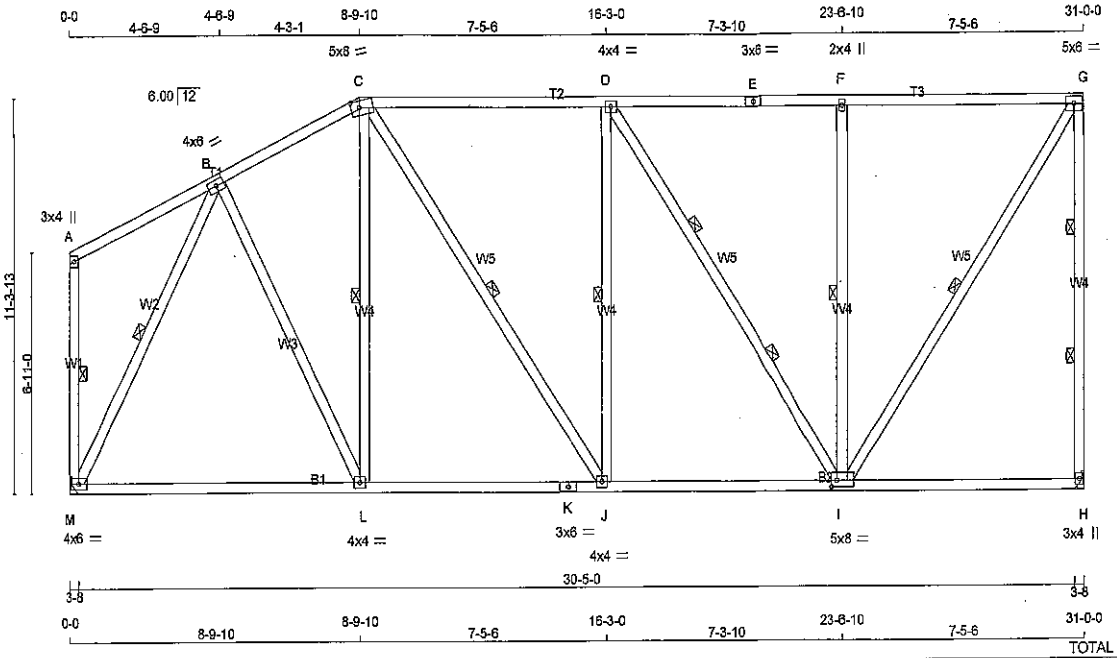


TOTAL WEIGHT = 2 X 194 = 388 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - A	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	6.0		
C	TTWW-m	MT20	5.0	8.0	2.25	3.00
D	TMWW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TMVW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWWW-t	MT20	5.0	8.0	2.25	2.00
J	BMWW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	1740	0	1740	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	
M	1740	0	1740	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1429	793 / 0	326 / 0	0 / 0	0 / 0	310 / 0	0 / 0
M	1429	793 / 0	326 / 0	0 / 0	0 / 0	310 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

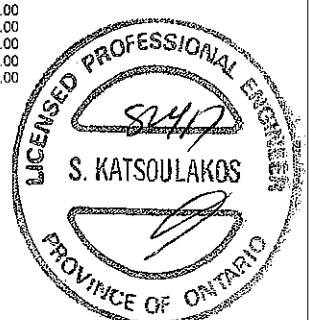
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2 LATERAL BRACE(S) AT 1/ 3 LENGTH OF G-H, D-I.
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-L, C-J, D-J, F-I, G-I, A-M, B-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 20	-84.3	-84.3	0.27 (1)	10.00	B- L	0 / 584	0.09 (1)
B- C	-1162 / 0	-84.3	-84.3	0.30 (1)	5.53	L- C	-252 / 95	0.17 (1)
C- D	-1269 / 0	-84.3	-84.3	0.76 (1)	4.58	C- J	0 / 452	0.07 (1)
D- E	-948 / 0	-84.3	-84.3	0.71 (1)	5.18	J- D	-197 / 170	0.13 (1)
E- F	-948 / 0	-84.3	-84.3	0.71 (1)	5.18	D- I	-581 / 0	0.25 (1)
F- G	-948 / 0	-84.3	-84.3	0.71 (1)	5.20	I- F	-681 / 0	0.46 (1)
H- G	-1657 / 0	0.0	0.0	0.48 (1)	5.15	I- G	0 / 1694	0.27 (1)
M- A	-145 / 0	0.0	0.0	0.03 (1)	6.25	M- B	-1675 / 0	0.82 (1)
M- L	0 / 757	-28.0	-28.0	0.60 (2)	10.00			
L- K	0 / 1017	-28.0	-28.0	0.64 (2)	10.00			
K- J	0 / 1017	-28.0	-28.0	0.64 (2)	10.00			
J- I	0 / 1269	-28.0	-28.0	0.54 (2)	10.00			
I- H	0 / 0	-28.0	-28.0	0.40 (3)	10.00			



DWG NO. TAM41909-17
STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.24")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 936 (0.40")

CSI: TC=0.76 (C-D:1) , BC=0.64 (J-L:2) , WB=0.82 (B-M:1) , SSI=0.29 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (M) (INPUT = 0.90)
JSI METAL= 0.34 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285351	G59	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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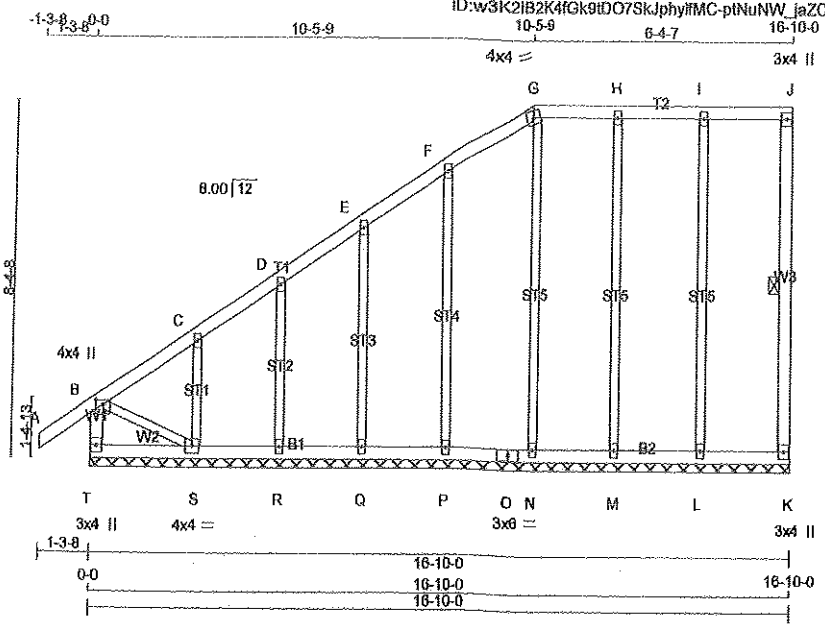

TOTAL WEIGHT = 89 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
T - B	2x4	DRY	No.2	SPF
A - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
T - O	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C, D, E, F, H, I						
C	TMW+w	MT20	2.0	4.0		
G	TTW-m	MT20	4.0	4.0		
J	TMV+p	MT20	3.0	4.0		
K	BMV1+p	MT20	3.0	4.0		
L, M, N, P, Q, R						
L	BMW1+w	MT20	2.0	4.0		
O	BS-t	MT20	3.0	6.0		
S	BMWW1-t	MT20	4.0	4.0		
T	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF J-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
T-B	-218 / 0	0.0	0.0	0.02 (1)		7.81
A-B	0 / 32	-84.3	-84.3	0.11 (1)		10.00
B-C	-8 / 0	-84.3	-84.3	0.07 (1)		10.00
C-D	-18 / 0	-84.3	-84.3	0.07 (1)		6.25
D-E	-7 / 0	-84.3	-84.3	0.04 (1)		10.00
E-F	-3 / 0	-84.3	-84.3	0.04 (1)		10.00
F-G	-11 / 0	-84.3	-84.3	0.04 (1)		6.25
G-H	0 / 0	-84.3	-84.3	0.05 (1)		10.00
H-I	0 / 0	-84.3	-84.3	0.05 (1)		10.00
I-J	0 / 0	-84.3	-84.3	0.05 (1)		10.00
K-J	-74 / 0	0.0	0.0	0.02 (1)		6.25
T-S	0 / 0	-28.0	-28.0	0.04 (2)		10.00
S-R	0 / 10	-28.0	-28.0	0.04 (2)		10.00
R-Q	0 / 6	-28.0	-28.0	0.02 (2)		10.00
Q-P	0 / 4	-28.0	-28.0	0.02 (2)		10.00
P-O	0 / 2	-28.0	-28.0	0.02 (2)		10.00
O-N	0 / 2	-28.0	-28.0	0.02 (2)		10.00
N-M	0 / 0	-28.0	-28.0	0.02 (2)		10.00
M-L	0 / 0	-28.0	-28.0	0.03 (2)		10.00
L-K	0 / 0	-28.0	-28.0	0.03 (2)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L-I	-156 / 0	0.27 (1)
M-H	-166 / 0	0.27 (1)
N-G	-137 / 0	0.19 (1)
P-F	-185 / 0	0.16 (1)
Q-E	-167 / 0	0.09 (1)
R-D	-152 / 0	0.04 (1)
S-C	-217 / 0	0.04 (1)
B-S	0 / 17	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCEC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 0.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11/1.00 (A-B:1) , BC=0.04/1.00 (R-S:2) , WB=0.27/1.00 (H-M:1) , SSI=0.08/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.56 (C) (INPUT = 0.90)
JSI METAL= 0.05 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
06/26/18
H.J.G. AMES
PROVINCE OF ONTARIO

DWG NO. TAM 180646
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285351	T60	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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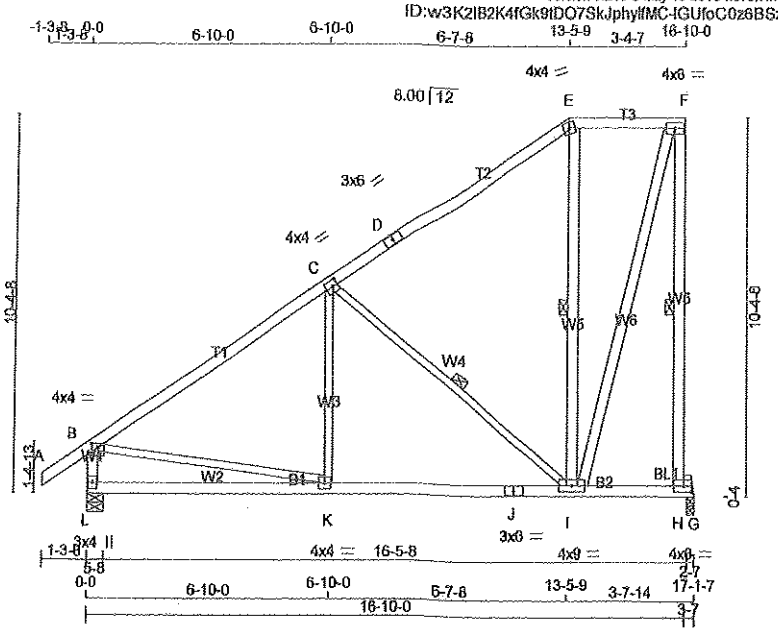

TOTAL WEIGHT = 96 lb (M)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
I - E	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	4.0	1.25	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTW-m	MT20	4.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
H	BMVK-t	MT20	4.0	6.0	2.00	2.75
I	BMWWW-t	MT20	4.0	9.0		
J	BS-t	MT20	3.0	8.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1077	0	1077	0	0	5-8	5-8
G	973	0	973	0	0	2-7	2-7

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	868	508 / 0	180 / 0	0 / 0	0 / 0	179 / 0	0 / 0
G	797	445 / 0	180 / 0	0 / 0	0 / 0	172 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.15 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-H, C-I, E-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	K-C	0 / 317	0.07 (3)
B-C	-929 / 0	-84.3	-84.3	0.75 (1)	5.15	C-I	-683 / 0	0.36 (1)
C-D	-376 / 0	-84.3	-84.3	0.67 (1)	6.25	I-E	-164 / 67	0.09 (1)
D-E	-376 / 0	-84.3	-84.3	0.67 (1)	6.25	I-F	0 / 982	0.15 (1)
E-F	-278 / 0	-84.3	-84.3	0.16 (1)	6.25	B-K	0 / 815	0.18 (1)
H-F	-1057 / 0	0.0	0.0	0.56 (1)	6.13			
L-B	-999 / 0	0.0	0.0	0.10 (1)	7.81			
L-K	0 / 0	-28.0	-28.0	0.33 (3)	10.00			
K-J	0 / 805	-28.0	-28.0	0.41 (2)	10.00			
J-I	0 / 805	-28.0	-28.0	0.41 (2)	10.00			
I-H	15 / 0	-28.0	-28.0	0.56 (1)	6.25			
E-I	0 / 0	-112.3	-112.3	0.23 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF	
	DL =	3.0	PSF	
BOT CH.	LL =	10.5	PSF	
	DL =	7.0	PSF	
TOTAL LOAD	=	46.1	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(65 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.57")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.57")
CALCULATED VERT. DEFL.(TL) = L/ 989 (0.15")

CSI: TC=0.75/1.00 (B-C:1) , BC=0.56/1.00 (H-I:1) , WB=0.36/1.00 (C-I:1) , SSI=0.36/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1067	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (B) (INPUT = 0.90)
JSI METAL= 0.31 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
06/26/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM 1800647
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285351	T61	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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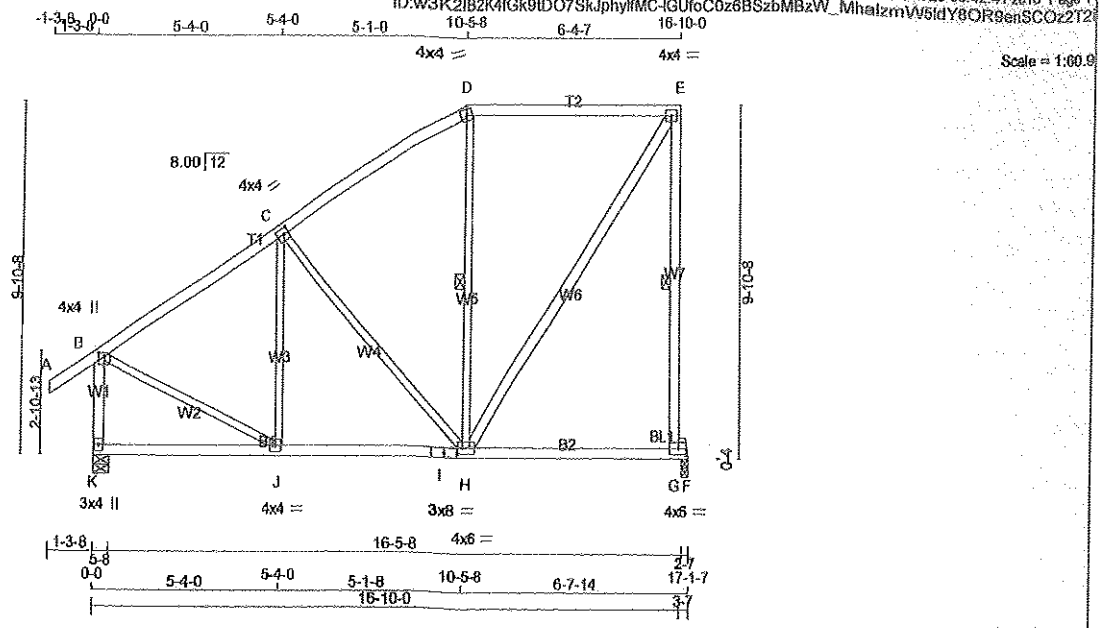

TOTAL WEIGHT = 92 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMVW-t	MT20	4.0	4.0		
G	BMVK-t	MT20	4.0	6.0	2.00	2.75
H	BMWWW-t	MT20	4.0	6.0	2.00	1.50
I	BS-t	MT20	3.0	8.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	1077	0	1077	0	0	5-8	5-8
F	973	0	973	0	0	2-7	2-7

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	868	508 / 0	180 / 0	0 / 0	0 / 0	179 / 0	0 / 0
F	797	445 / 0	160 / 0	0 / 0	0 / 0	172 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	
B- C	-725 / 0	-84.3	-84.3	0.42 (1)	6.25	
C- D	-545 / 0	-84.3	-84.3	0.41 (1)	6.25	
D- E	-426 / 0	-84.3	-84.3	0.60 (1)	6.25	
G- E	-940 / 0	0.0	0.0	0.44 (1)	6.25	
K- B	-1009 / 0	0.0	0.0	0.16 (1)	7.79	
K- J	0 / 0	-28.0	-28.0	0.17 (2)	10.00	
J- I	0 / 628	-28.0	-28.0	0.42 (2)	10.00	
I- H	0 / 628	-28.0	-28.0	0.42 (2)	10.00	
H- G	-14 / 0	-28.0	-28.0	0.56 (1)	6.25	
G- F	0 / 0	-112.3	-112.3	0.23 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- C	-181 / 107	0.12 (1)
C- H	-309 / 0	0.42 (1)
H- D	-144 / 113	0.09 (1)
H- E	0 / 804	0.13 (1)
B- J	0 / 097	0.10 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.57")
CALCULATED VERT. DEFL.(LL) = L/ 899 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (0.57")
CALCULATED VERT. DEFL.(TL) = L/ 703 (0.29")

CSI: TC=0.60/1.00 (D-E:1) , BC=0.56/1.00 (G-H:1) , WB=0.42/1.00 (C-H:1) , SSI=0.38/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1687	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (G) (INPUT = 0.90)
JSI METAL= 0.51 (I) (INPUT = 1.00)

DWG NO. TAM 1800648
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285351	T62	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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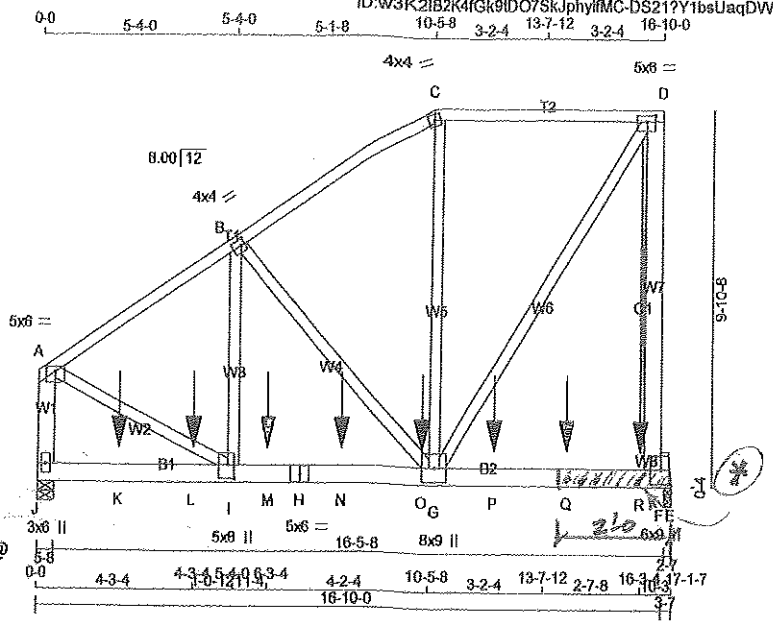

Scale = 1:57.2

PROVIDE 24" LONG SCAB 2X6 SPF #2 ALONG MENBERS G-F, ON ONE SIDE, USING 2 ROWS OF 3 1/2" SPIRAL NAILS @ 4" C/C. (12 NAILS MIN)

TOTAL WEIGHT = 3 X 117 = 352 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
J - A	2x6	DRY	No.2	SPF
J - H	2x6	DRY	2100F 1.8E	SPF
H - E	2x6	DRY	2100F 1.8E	SPF
F - E	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- D	1	12	TOP
D- F	2	12	SIDE(88.8)
J- A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
J- H	2	9	SIDE(476.4)
H- E	2	9	SIDE(494.5)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	
2x6	2	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-p	MT20	5.0	6.0	1.50	3.00
B	TMWW-t	MT20	4.0	4.0	2.00	1.00
C	TTW-m	MT20	4.0	4.0		
D	TMWW-t	MT20	5.0	8.0	2.00	2.75
F	BMWW+p	MT20	6.0	9.0	4.50	2.50
G	BMWWW+t	MT20	8.0	9.0		
H	BS-t	MT20	5.0	6.0		
I	BMWW+t	MT20	5.0	8.0	4.25	2.00
J	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES
1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	7398	0	7398	0	0	5-8	5-8
E	8426	0	8426	0	0	2-7	3-7

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	6008	3443 / 0	1297 / 0	0 / 0	0 / 0	1269 / 0	0 / 0
E	6801	3902 / 0	1500 / 0	0 / 0	0 / 0	1458 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.48 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x6 DRY SPF No.2 T-BRACE AT D-F

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A-B	-6507 / 0	-84.3	-84.3	0.23 (1)		4.48
B-C	-4748 / 0	-84.3	-84.3	0.17 (1)		5.11
C-D	-3955 / 0	-84.3	-84.3	0.25 (1)		5.38
F-D	-6283 / 0	0.0	0.0	0.59 (1)		7.01
J-A	-8346 / 0	0.0	0.0	0.20 (1)		6.99
J-K	0 / 0	-28.0	-28.0	0.28 (1)		10.00
K-L	0 / 0	-28.0	-28.0	0.28 (1)		10.00
L-I	0 / 0	-28.0	-28.0	0.26 (1)		10.00
I-M	0 / 5436	-28.0	-28.0	0.54 (1)		10.00
M-H	0 / 5436	-28.0	-28.0	0.54 (1)		10.00
H-N	0 / 5436	-28.0	-28.0	0.54 (1)		10.00
N-O	0 / 5436	-28.0	-28.0	0.54 (1)		10.00
O-G	0 / 5436	-28.0	-28.0	0.54 (1)		10.00
G-P	0 / 0	-28.0	-28.0	0.53 (1)		10.00
P-Q	0 / 0	-28.0	-28.0	0.53 (1)		10.00
Q-R	0 / 0	-28.0	-28.0	0.53 (1)		10.00
R-F	0 / 0	-28.0	-28.0	0.53 (1)		10.00
F-E	0 / 0	-28.0	-28.0	0.22 (1)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I-B	0 / 2202	0.42 (1)
B-G	-2306 / 0	0.72 (1)
G-C	0 / 2184	0.42 (1)
G-D	0 / 1214	0.39 (1)
A-I	0 / 6002	0.92 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE
K	2-3-4	-1394	-1394	---	BACK	VERT	TOTAL
L	4-3-4	-1471	-1471	---	BACK	VERT	TOTAL
M	6-3-4	-1471	-1471	---	BACK	VERT	TOTAL
N	8-3-4	-1394	-1394	---	BACK	VERT	TOTAL
O	10-3-4	-1394	-1394	---	BACK	VERT	TOTAL
P	12-3-4	-1394	-1394	---	BACK	VERT	TOTAL
Q	14-3-4	-1394	-1394	---	BACK	VERT	TOTAL

DESIGN CRITERIA [M]

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.57")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.57")
CALCULATED VERT. DEFL.(TL) = L/ 778 (0.26")

CSI: TC=0.59/1.00 (D-F:1) , BC=0.54/1.00 (G-I:1) , WB=0.72/1.00 (B-G:1) , SSI=0.85/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (D) (INPUT = 0.90)
JSI METAL= 0.47 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
06/26/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM T18006419
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285351	T62	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1711.9 lbs FACTORED DOWN AT 2-3-4, 1826.2 lbs FACTORED DOWN AT 4-3-4, 1826.2 lbs FACTORED DOWN AT 6-3-4, 1711.9 lbs FACTORED DOWN AT 8-3-4, 1711.9 lbs FACTORED DOWN AT 10-3-4, 1711.9 lbs FACTORED DOWN AT 12-3-4, AND 1711.9 lbs FACTORED DOWN AT 14-3-4, AND 1714.6 lbs FACTORED DOWN AT 16-3-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
R	16-3-4	-1398	-1396	---	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
06/26/18
H.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM T1900644
STRUCTURAL
COMPONENT ONLY

2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286072	T63	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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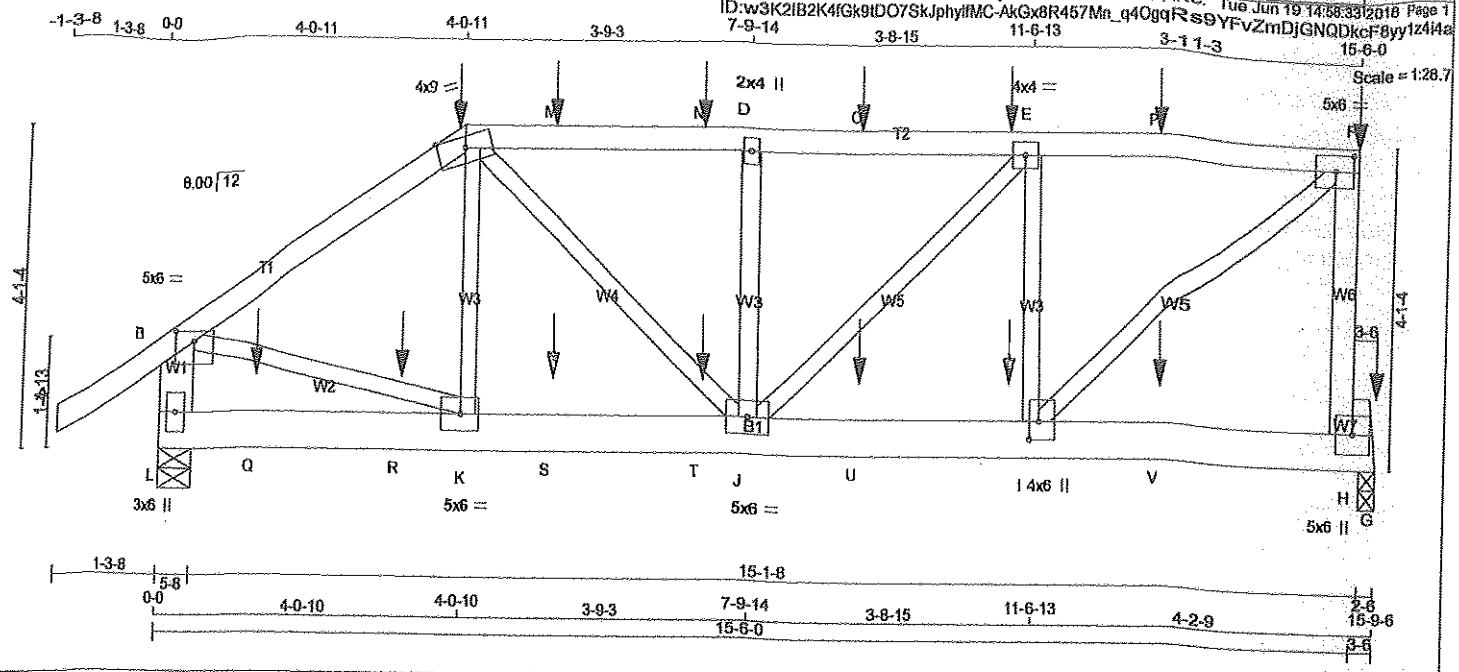


TOTAL WEIGHT = 78 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - B	2x6	DRY	No.2	SPF
L - G	2x6	DRY	No.2	SPF
H - G	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TTWW-m	MT20	4.0	9.0	Edge	
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMVW-t	MT20	5.0	6.0	2.50	2.75
H	BMVW+p	MT20	5.0	6.0		
I	BMWW+t	MT20	4.0	6.0	2.75	1.50
J	BMWWW-t	MT20	5.0	6.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 244.5 lbs FACTORED DOWN AT 4-0-11, 101.4 lbs FACTORED DOWN AT 5-3-4, 101.4 lbs FACTORED DOWN AT 7-3-4, 101.4 lbs FACTORED DOWN AT 9-3-4, 101.4 lbs FACTORED DOWN AT 11-3-4, AND 101.4 lbs FACTORED DOWN AT 13-3-4, AND 132.1 lbs FACTORED DOWN AT 15-6-0 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 1-3-4, 69.9 lbs FACTORED DOWN AT 3-3-4, 69.9 lbs FACTORED DOWN AT 5-3-4, 69.9 lbs FACTORED DOWN AT 7-3-4, 69.9 lbs FACTORED DOWN AT 9-3-4, 69.9 lbs FACTORED DOWN AT 11-3-4, AND 69.9 lbs FACTORED DOWN AT 13-3-4, AND 67.8 lbs FACTORED DOWN AT 15-9-6 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS 15-9-6

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
L	1543	0	1543	0	0	5-8	5-8
G	1533	0	1533	0	0	2-6	2-6

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1260	711 / 0	279 / 0	0 / 0	0 / 0	270 / 0	0 / 0
G	1269	688 / 0	300 / 0	0 / 0	0 / 0	281 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.80 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.3	-84.3	0.12 (1)	10.00
B-C	-1559 / 0	-84.3	-84.3	0.32 (1)	4.80
C-M	-1624 / 0	-84.3	-84.3	0.31 (1)	4.80
M-N	-1624 / 0	-84.3	-84.3	0.31 (1)	4.80
N-D	-1624 / 0	-84.3	-84.3	0.31 (1)	4.80
D-O	-1624 / 0	-84.3	-84.3	0.31 (1)	4.81
O-E	-1624 / 0	-84.3	-84.3	0.31 (1)	4.81
E-P	-1315 / 0	-84.3	-84.3	0.28 (1)	5.27
P-F	-1315 / 0	-84.3	-84.3	0.28 (1)	5.27
H-F	-1546 / 0	0.0	0.0	0.39 (1)	6.58
L-B	-1465 / 0	0.0	0.0	0.10 (1)	7.81
L-Q	0 / 0	-28.0	-28.0	0.10 (2)	10.00
Q-R	0 / 0	-28.0	-28.0	0.10 (2)	10.00
R-K	0 / 0	-28.0	-28.0	0.10 (2)	10.00
K-S	0 / 1292	-28.0	-28.0	0.23 (1)	10.00
S-T	0 / 1292	-28.0	-28.0	0.23 (1)	10.00
T-J	0 / 1292	-28.0	-28.0	0.23 (1)	10.00
J-U	0 / 1346	-28.0	-28.0	0.27 (1)	10.00
U-I	0 / 1346	-28.0	-28.0	0.27 (1)	10.00
I-V	0 / 0	-28.0	-28.0	0.32 (1)	10.00
V-H	0 / 0	-28.0	-28.0	0.32 (1)	10.00
H-G	0 / 0	-28.0	-28.0	0.13 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
K-C	-77 / 226	0.08 (3)
B-K	0 / 1331	0.33 (1)
I-F	0 / 1612	0.45 (1)
C-J	0 / 466	0.12 (1)
I-E	-869 / 0	0.22 (1)
J-D	-540 / 0	0.14 (1)
J-E	0 / 436	0.11 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE
C	4-0-11	-148	-148	---	FRONT	VERT	DEAD
C	4-0-11	-148	-148	---	FRONT	VERT	SNOW
M	5-3-4	-69	-69	---	FRONT	VERT	TOTAL
P	15-6-0	-90	-90	---	FRONT	VERT	TOTAL
H	15-9-6	-83	-83	---	FRONT	VERT	TOTAL
I	11-3-4	-50	-50	---	FRONT	VERT	TOTAL
M	5-3-4	-69	-69	---	FRONT	VERT	TOTAL
N	7-3-4	-69	-69	---	FRONT	VERT	TOTAL
O	9-3-4	-69	-69	---	FRONT	VERT	TOTAL
P	13-3-4	-69	-69	---	FRONT	VERT	TOTAL
Q	1-3-4	-50	-50	---	FRONT	VERT	TOTAL
R	3-3-4	-50	-50	---	FRONT	VERT	TOTAL
S	5-3-4	-50	-50	---	FRONT	VERT	TOTAL
T	7-3-4	-50	-50	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	48.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.53")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.53")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.39/1.00 (F-H:1) , BC=0.32/1.00 (H-I:1) , WB=0.45/1.00 (F-I:1) , SSI=0.51/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

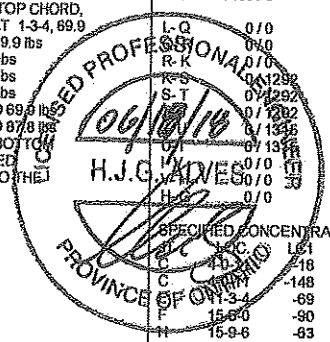
PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1697	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.69 (C) @INPUT = 0.90)
JSI METAL= 0.43 (I) @INPUT = 1.00)

CONTINUED ON PAGE 2



T1800542-1

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286072	T63	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
U	9-3-4	-50	-50	—	FRONT	VERT	TOTAL
V	13-3-4	-50	-50	—	FRONT	VERT	TOTAL

T1800542-2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286072	T64	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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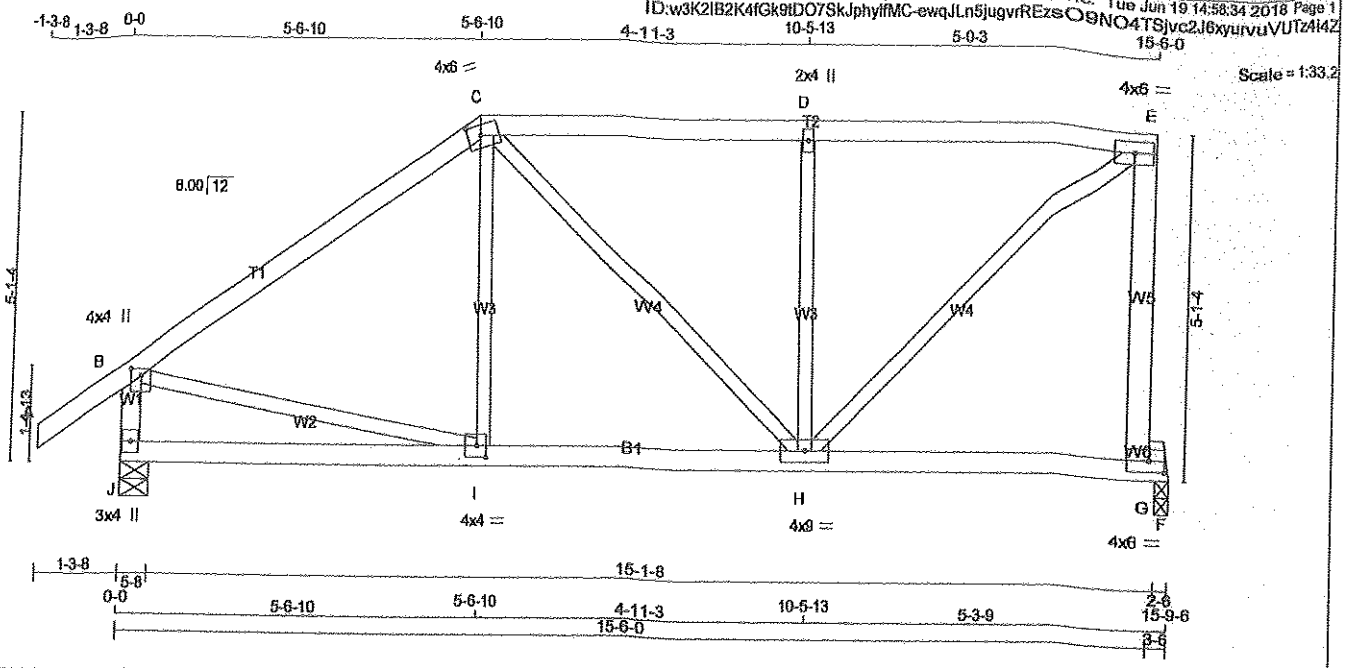


TOTAL WEIGHT = 68 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
J - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTWW-m	MT20	4.0	6.0	1.75	2.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
G	BMVW-t	MT20	4.0	6.0	2.00	2.50
H	BMWWW-t	MT20	4.0	9.0		
I	BMWW-t	MT20	4.0	4.0	2.00	1.75
J	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1001	0	1001	0	0	5-8	5-8
F	882	0	882	0	0	2-6	2-6

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	806	474 / 0	166 / 0	0 / 0	0 / 0	166 / 0	0 / 0
F	711	389 / 0	166 / 0	0 / 0	0 / 0	156 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.97 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

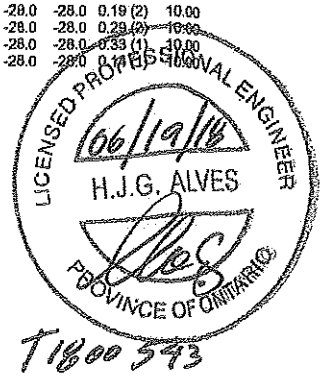
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.3	-84.3	1	0.11	10.00
B- C	-916 / 0	-84.3	-84.3	1	0.49	5.97
C- D	-723 / 0	-84.3	-84.3	1	0.36	6.25
D- E	-723 / 0	-84.3	-84.3	1	0.36	6.25
G- E	-856 / 0	0.0	0.0	1	0.38	7.81
J- B	-933 / 0	0.0	0.0	1	0.10	7.81
J- I	0 / 0	-28.0	-28.0	2	0.19	10.00
I- H	0 / 679	-28.0	-28.0	2	0.29	10.00
H- G	0 / 0	-28.0	-28.0	2	0.33	10.00
G- F	0 / 0	-28.0	-28.0	2	0.19	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI
I- C	0 / 210	0.05 (3)
C- H	-8 / 62	0.01 (1)
H- D	-517 / 0	0.20 (1)
H- E	0 / 1003	0.23 (1)
B- I	0 / 691	0.16 (1)



T1800543

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	45.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.53")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.53")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.49/1.00 (B-C:1) , BC=0.33/1.00 (G-H:1) , WB=0.23/1.00 (E-H:1) , SSI=0.34/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1056

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (G) (INPUT = 0.90)
JSI METAL= 0.26 (G) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286072	T65	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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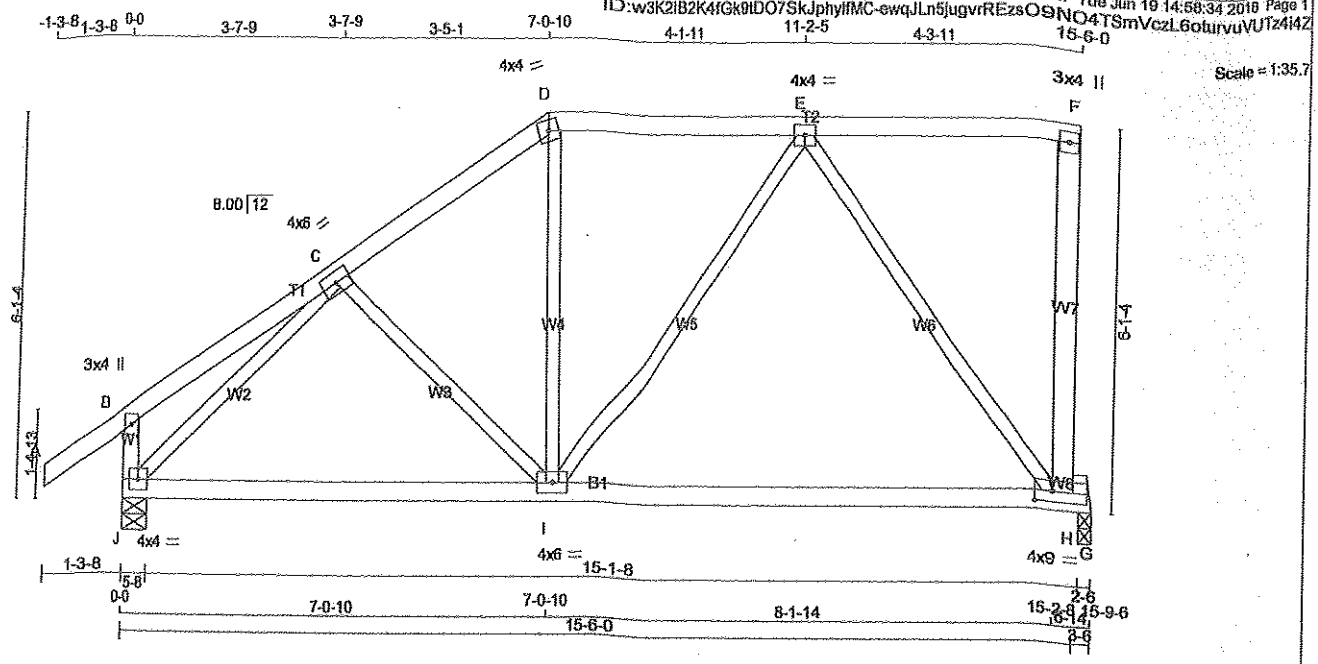

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVWW-t	MT20	4.0	9.0	2.00	3.00
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	862	0	862	0	0	2-6	2-6
J	1001	0	1001	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	711	389 / 0	166 / 0	0 / 0	0 / 0	156 / 0	0 / 0
J	806	474 / 0	166 / 0	0 / 0	0 / 0	166 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00
B- C	0 / 18	-84.3	-84.3	0.16 (1)	10.00
C- D	-787 / 0	-84.3	-84.3	0.17 (1)	6.25
D- E	-642 / 0	-84.3	-84.3	0.26 (1)	6.25
E- F	0 / 0	-84.3	-84.3	0.25 (1)	10.00
H- F	-139 / 0	0.0	0.0	0.10 (1)	7.81
J- B	-231 / 0	0.0	0.0	0.02 (1)	7.81
J- I	0 / 726	-28.0	-28.0	0.65 (2)	10.00
I- H	0 / 469	-28.0	-28.0	0.62 (2)	10.00
H- G	0 / 0	-28.0	-28.0	0.14 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- I	-122 / 58	0.05 (1)
I- D	0 / 238	0.05 (2)
J- C	-1015 / 0	0.39 (1)
I- E	0 / 323	0.07 (2)
E- H	-821 / 0	0.01 (1)

LICENSED PROFESSIONAL ENGINEER
06/19/18
H.J.G. ALVES
PROVINCE OF ONTARIO
T18005444

TOTAL WEIGHT = 70 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.53")
CALCULATED VERT. DEFL.(LL) = L/ 713 (0.27")
ALLOWABLE DEFL.(TL)= L/360 (0.53")
CALCULATED VERT. DEFL.(TL) = L/ 433 (0.44")

CSI: TC=0.26/1.00 (D-E:1) , BC=0.65/1.00 (I-J:2) , WB=0.61/1.00 (E-H:1) , SSI=0.34/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (J) (INPUT = 0.90)
JSI METAL= 0.27 (J) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285351	T66	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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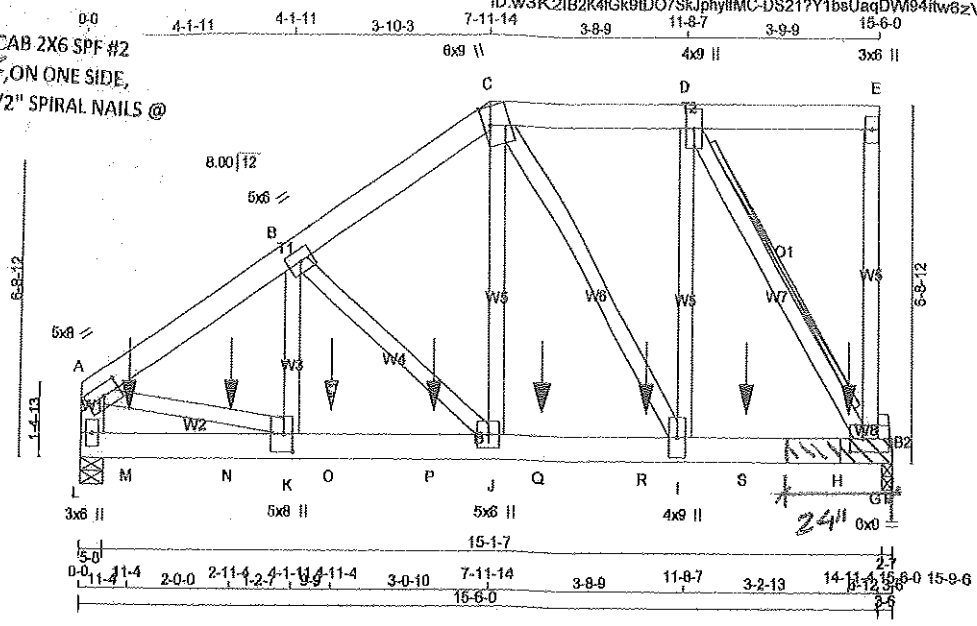
ID:w3K2IB2K4fGk9tDO7SkJphyfMC-DS21?Y1bsUaqDW94itw6zV2bV6AHaObNiW?kqz212h

PROVIDE 24" LONG SCAB 2X6 SPF #2 ALONG MEMBERS D-E, ON ONE SIDE, USING 2 ROWS OF 3 1/2" SPIRAL NAILS @ 4" C/C. (12 NAILS MIN)



TOTAL WEIGHT = 3 X 112 = 335 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
L - A	2x6	DRY	No.2	SPF
L - F	2x8	DRY	2100F 1.8E	SPF
G - F	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	TOP
C- E	2	12	TOP
L- A	2	12	TOP
E- G	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
L- F	2	9	SIDE(803.7)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	
2x6	2	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	8.0	2.50	3.75
B	TMWW-t	MT20	5.0	6.0		
C	TTWW+m	MT20	6.0	9.0	4.25	1.50
D	TMWW+t	MT20	4.0	9.0		
E	TMV+p	MT20	3.0	6.0		
G	BMVWW-t	MT20	6.0	9.0	3.00	3.00
I	BMWW+t	MT20	4.0	9.0		
J	BMWW+t	MT20	5.0	6.0		
K	BMWW+t	MT20	5.0	6.0	4.00	2.00
L	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	7882	0	7882	0	0	5-8	5-8
F	7793	0	7793	0	0	2-7	2-7 & BLOCK

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	6400	3669 / 0	1380 / 0	0 / 0	0 / 0	1350 / 0	0 / 0
F	6348	3608 / 0	1389 / 0	0 / 0	0 / 0	1349 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, F

2x6 DRY SPF #2 : BEARING BLOCK 24" LONG AT JT. F ATTACHED TO FRONT SIDE WITH 2 ROWS OF (0.122"X3") SPIRAL NAILS SPACED 4" C.C. 12 NAILS TOTAL.

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.96 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x6 DRY SPF No.2 T-BRACE AT D-G

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF)	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-6356 / 0	-84.3	-84.3	0.09 (1)	4.96	
B- C	-5685 / 0	-84.3	-84.3	0.07 (1)	5.67	
C- D	-4034 / 0	-84.3	-84.3	0.03 (1)	6.25	
D- E	0 / 0	-84.3	-84.3	0.02 (1)	10.00	
G- E	-135 / 0	0.0	0.0	0.04 (1)	7.81	
L- A	-6329 / 0	0.0	0.0	0.14 (1)	7.00	
L- M	0 / 0	-28.0	-28.0	0.18 (1)	10.00	
M- N	0 / 0	-28.0	-28.0	0.18 (1)	10.00	
N- K	0 / 0	-28.0	-28.0	0.18 (1)	10.00	
K- O	0 / 6940	-28.0	-28.0	0.29 (1)	10.00	
O- P	0 / 6940	-28.0	-28.0	0.29 (1)	10.00	
P- J	0 / 6940	-28.0	-28.0	0.29 (1)	10.00	
J- Q	0 / 5071	-28.0	-28.0	0.30 (1)	10.00	
Q- R	0 / 5071	-28.0	-28.0	0.30 (1)	10.00	
R- I	0 / 5071	-28.0	-28.0	0.30 (1)	10.00	
I- S	0 / 4084	-28.0	-28.0	0.41 (1)	10.00	
S- H	0 / 4084	-28.0	-28.0	0.41 (1)	10.00	
H- G	0 / 4084	-28.0	-28.0	0.41 (1)	10.00	
G- F	0 / 0	-28.0	-28.0	0.20 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- B	0 / 2817	0.45 (1)
B- J	-2682 / 0	0.29 (1)
J- C	0 / 4698	0.25 (1)
C- I	-1572 / 0	0.43 (1)
I- D	0 / 6374	0.34 (1)
D- G	-7649 / 0	0.60 (1)
A- K	0 / 7101	0.38 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
H	14-11-4	-1398	-1398	—	FRONT	VERT	TOTAL
M	11-4	-1395	-1395	—	FRONT	VERT	TOTAL
N	2-11-4	-1471	-1471	—	FRONT	VERT	TOTAL
O	4-11-4	-1471	-1471	—	FRONT	VERT	TOTAL
P	6-11-4	-1394	-1394	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- YPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.53")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.14/1.00 (A-L:1) , BC=0.41/1.00 (G-I:1) , WB=0.50/1.00 (D-G:1) , SSI=0.76/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	822	2284	1656

(PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

MSI GRIP= 0.86 (G) (INPUT = 0.90)
MSI METAL= 0.80 (K) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H.J.G. ALVES
PROVINCE OF ONTARIO
Jun 26/18

DWG NO. TAM T1800650
STRUCTURAL COMPONENT ONLY

1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285351	T66	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1712.8 lbs FACTORED DOWN AT 11-4, 1826.2 lbs FACTORED DOWN AT 2-11-4, 1826.2 lbs FACTORED DOWN AT 4-11-4, 1711.9 lbs FACTORED DOWN AT 6-11-4, 1711.9 lbs FACTORED DOWN AT 8-11-4, 1711.9 lbs FACTORED DOWN AT 10-11-4, AND 1711.9 lbs FACTORED DOWN AT 12-11-4, AND 1714.1 lbs FACTORED DOWN AT 14-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
Q	6-11-4	-1394	-1394	---	FRONT	VERT	TOTAL
R	10-11-4	-1394	-1394	---	FRONT	VERT	TOTAL
S	12-11-4	-1394	-1394	---	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
06/26/18
P.J.G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM T1800650
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285348	T80	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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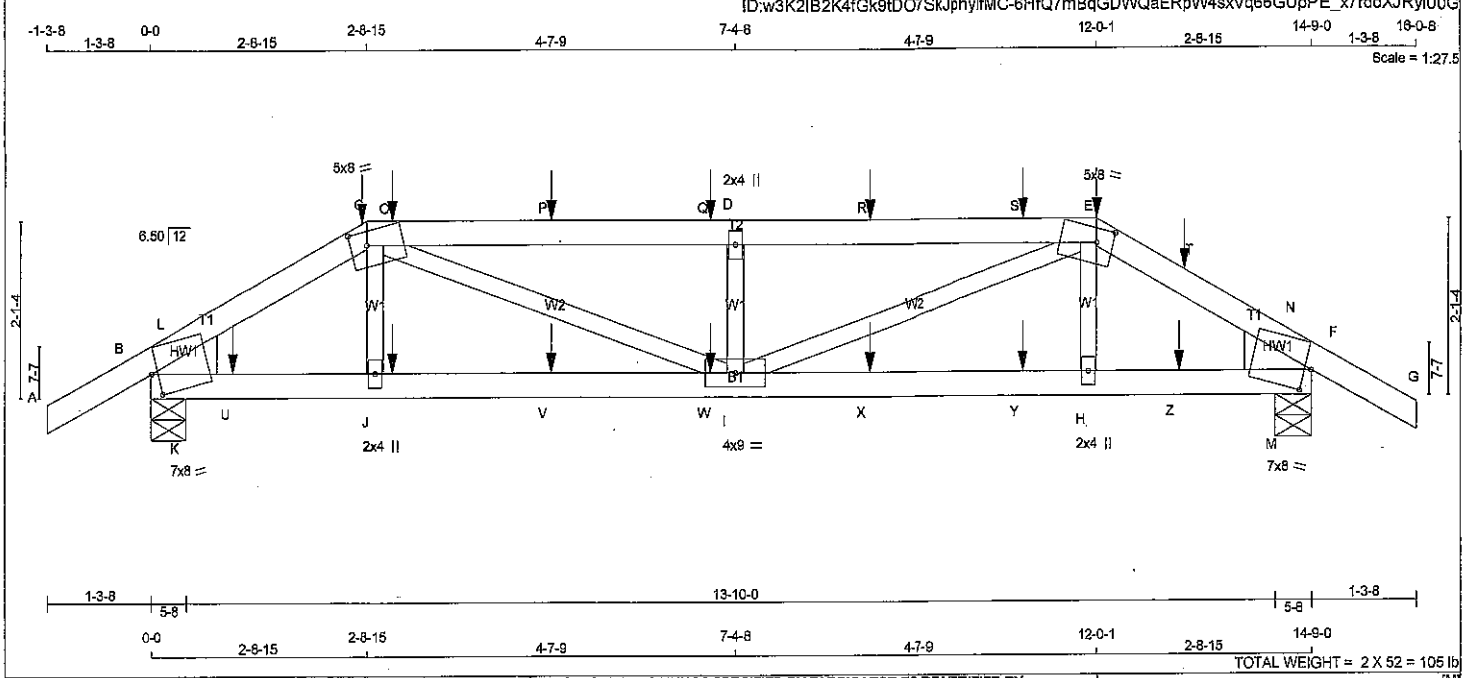


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-m	MT20	7.0	8.0	3.25	1.00
C	TTWW-m	MT20	5.0	8.0	2.00	2.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW-m	MT20	5.0	8.0	2.00	2.50
F	TMBH1-m	MT20	7.0	8.0	3.25	1.00
H	BMW+w	MT20	2.0	4.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BMW+w	MT20	2.0	4.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 60.3 lbs FACTORED DOWN AT 2-8-15, 60.3 lbs FACTORED DOWN AT 12-0-1, 144.6 lbs FACTORED DOWN AT 3-0-12, 115.9 lbs FACTORED DOWN AT 5-0-12, 115.9 lbs FACTORED DOWN AT 7-0-12, 115.9 lbs FACTORED DOWN AT 9-0-12, AND 118.3 lbs FACTORED DOWN AT 11-0-12, AND 115.9 lbs FACTORED DOWN AT 13-0-12 ON TOP CHORD, AND 76.0 lbs FACTORED DOWN AT 1-0-12, 76.0 lbs FACTORED DOWN AT 3-0-12, 76.0 lbs FACTORED DOWN AT 5-0-12, 76.0 lbs FACTORED DOWN AT 7-0-12, 76.0 lbs FACTORED DOWN AT 9-0-12, AND 76.0 lbs FACTORED DOWN AT 11-0-12, AND 76.0 lbs FACTORED DOWN AT 13-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
B	1602	0	1602	0	0	5-8	5-8	2x6 L
F	1628	0	1628	0	0	5-8	5-8	2x6 R

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	1297	750 / 0	275 / 0	0 / 0	0 / 0	272 / 0	0 / 0
F	1312	769 / 0	271 / 0	0 / 0	0 / 0	272 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.96 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 17	-84.3	-84.3	0.12 (1)	10.00
B-L	-2783 / 0	-84.3	-84.3	0.43 (1)	3.83
L-C	-2284 / 0	-84.3	-84.3	0.41 (1)	4.02
C-O	-3386 / 0	-84.3	-84.3	0.72 (1)	2.98
O-P	-3388 / 0	-84.3	-84.3	0.72 (1)	2.98
P-Q	-3386 / 0	-84.3	-84.3	0.72 (1)	2.98
Q-D	-3386 / 0	-84.3	-84.3	0.72 (1)	2.98
D-R	-3386 / 0	-84.3	-84.3	0.72 (1)	2.98
R-S	-3386 / 0	-84.3	-84.3	0.72 (1)	2.98
S-E	-3386 / 0	-84.3	-84.3	0.72 (1)	2.98
E-T	-2295 / 0	-84.3	-84.3	0.44 (1)	3.95
T-N	-2295 / 0	-84.3	-84.3	0.44 (1)	3.95
N-F	-2756 / 0	-84.3	-84.3	0.46 (1)	3.58
F-G	0 / 17	-84.3	-84.3	0.12 (1)	10.00
B-K	0 / 1984	-28.0	-28.0	0.67 (1)	10.00
K-U	0 / 1984	-28.0	-28.0	0.67 (1)	10.00
U-J	0 / 1984	-28.0	-28.0	0.67 (1)	10.00
J-V	0 / 1971	-28.0	-28.0	0.53 (1)	10.00
V-W	0 / 1971	-28.0	-28.0	0.53 (1)	10.00
W-I	0 / 1971	-28.0	-28.0	0.53 (1)	10.00
I-X	0 / 1981	-28.0	-28.0	0.51 (1)	10.00
X-Y	0 / 1981	-28.0	-28.0	0.51 (1)	10.00
Y-H	0 / 1981	-28.0	-28.0	0.51 (1)	10.00
H-Z	0 / 1973	-28.0	-28.0	0.64 (1)	10.00
Z-M	0 / 1973	-28.0	-28.0	0.64 (1)	10.00
M-F	0 / 1973	-28.0	-28.0	0.64 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J-C	0 / 244	0.06 (3)
C-I	0 / 1519	0.38 (1)
I-D	-784 / 0	0.13 (1)
I-E	0 / 1508	0.37 (1)
H-E	0 / 249	0.06 (3)
K-L	0 / 717	0.00 (1)
M-N	0 / 809	0.00 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-8-15	-6	-7	---	FRONT	VERT	DEAD
C	2-8-15	-54	-54	---	FRONT	VERT	SNOW
E	12-0-1	-6	-7	---	FRONT	VERT	DEAD
E	12-0-1	-54	-54	---	FRONT	VERT	SNOW
J	3-0-12	-71	-76	---	FRONT	VERT	TOTAL
O	3-0-12	-145	-145	---	FRONT	VERT	TOTAL
P	5-0-12	-116	-116	---	FRONT	VERT	TOTAL
Q	7-0-12	-116	-116	---	FRONT	VERT	TOTAL
R	9-0-12	-116	-116	---	FRONT	VERT	TOTAL
S	11-0-12	-118	-118	---	FRONT	VERT	TOTAL

[M]

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.49")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.49")
CALCULATED VERT. DEFL.(TL) = L/ 855 (0.21")

CSI: TC=0.72 (D-E:1) , BC=0.67 (B-K:1) , WB=0.38 (C-I:1) , SSI=0.46 (B-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

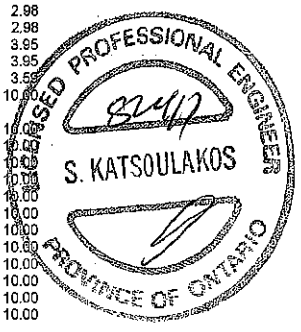
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	518	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (E) (INPUT = 0.90)
JSI METAL= 0.51 (E) (INPUT = 1.00)



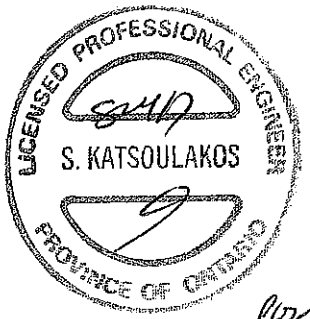
DWG NO. T&M 4197-17

STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
T	13-0-12	-116	-116	—	FRONT	VERT	TOTAL
U	1-0-12	-71	-76	—	FRONT	VERT	TOTAL
V	5-0-12	-71	-76	—	FRONT	VERT	TOTAL
W	7-0-12	-71	-76	—	FRONT	VERT	TOTAL
X	9-0-12	-71	-76	—	FRONT	VERT	TOTAL
Y	11-0-12	-71	-76	—	FRONT	VERT	TOTAL
Z	13-0-12	-71	-76	—	FRONT	VERT	TOTAL



DWG NO. TAM-41917-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285348	T93	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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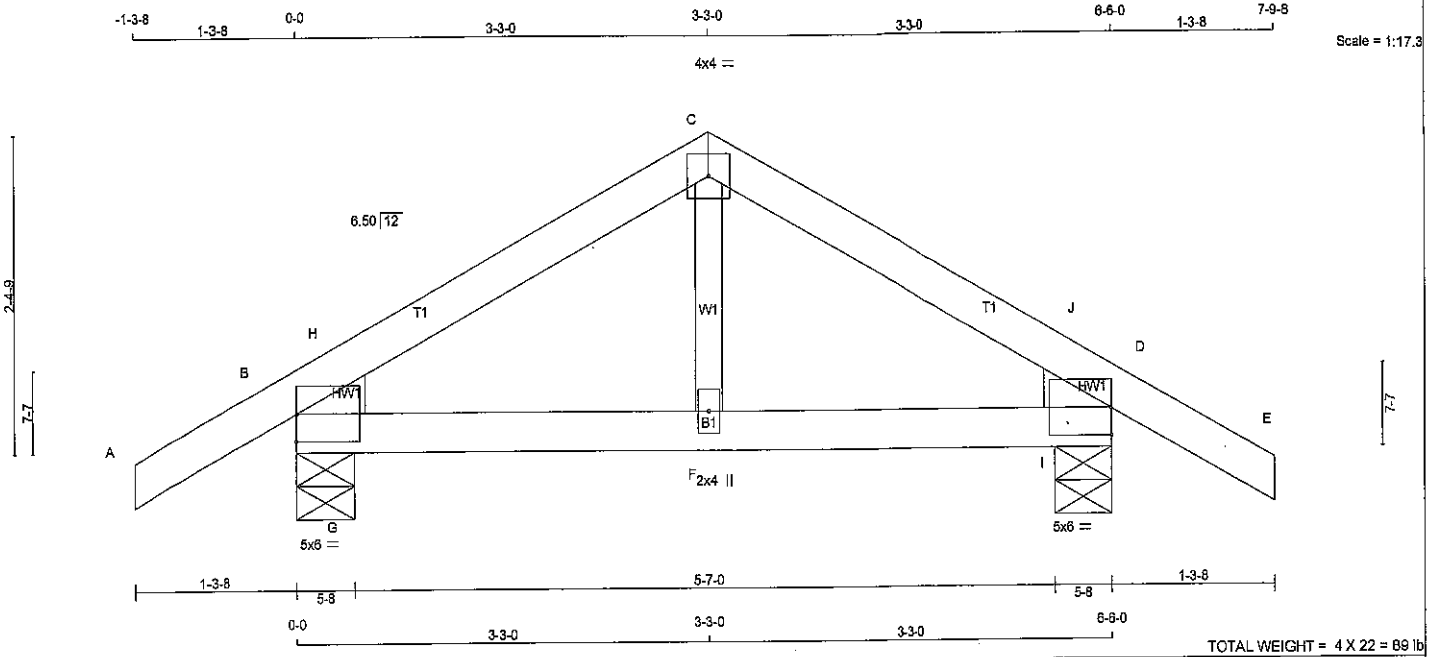

TOTAL WEIGHT = 4 X 22 = 89 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMBH1-I	MT20	5.0	6.0	
C	TTW-p	MT20	4.0	4.0	
D	TMBH1-I	MT20	5.0	6.0	
F	BMW+w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
B	480	0	480	0	0	5-8	5-8	2x4 L
D	480	0	480	0	0	5-8	5-8	2x4 R

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	377	236 / 0	68 / 0	0 / 0	0 / 0	73 / 0	0 / 0
D	377	236 / 0	68 / 0	0 / 0	0 / 0	73 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 17	-84.3	-84.3	0.11 (1)	10.00	F- C	0 / 165	0.04 (2)
B- H	-412 / 0	-84.3	-84.3	0.05 (1)	6.25	G- H	-34 / 113	0.00 (1)
H- C	-355 / 0	-84.3	-84.3	0.08 (1)	6.25	I- J	-34 / 113	0.00 (1)
C- J	-355 / 0	-84.3	-84.3	0.08 (1)	6.25			
J- D	-412 / 0	-84.3	-84.3	0.05 (1)	6.25			
D- E	0 / 17	-84.3	-84.3	0.11 (1)	10.00			
B- G	0 / 305	-28.0	-28.0	0.08 (1)	10.00			
G- F	0 / 305	-28.0	-28.0	0.10 (1)	10.00			
F- I	0 / 305	-28.0	-28.0	0.10 (1)	10.00			
I- D	0 / 305	-28.0	-28.0	0.08 (1)	10.00			

DWG NO. TAM 4/9/17-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.11 (A-B:1) , BC=0.10 (F-G:1) , WB=0.04 (C-F:2) , SSI=0.09 (C-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1657	822	2284	1856

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (D) (INPUT = 0.90)
JSI METAL= 0.10 (D) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285348	T93Z	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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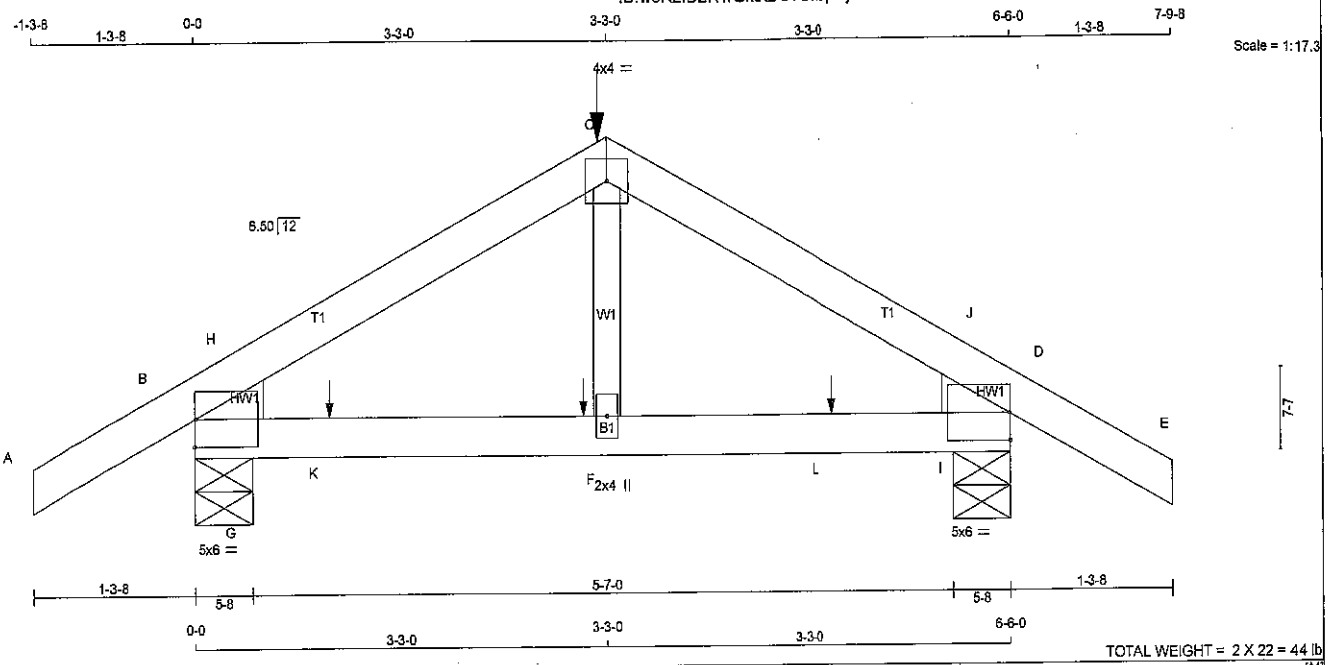

TOTAL WEIGHT = 2 X 22 = 44 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMBH1-I	MT20	5.0	6.0	
C	TTW-p	MT20	4.0	4.0	
D	TMBH1-I	MT20	5.0	6.0	
F	BMW+w	MT20	2.0	4.0	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 2.5 lbs FACTORED DOWN AT 3-3-0 ON TOP CHORD, AND AT 1-0-12, AND AT 3-0-12, AND AT 5-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
B	481	0	481	0	0	5-8	5-8	2x4 L
D	481	0	481	0	0	5-8	5-8	2x4 R

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	378	236 / 0	68 / 0	0 / 0	0 / 0	74 / 0	0 / 0
D	378	236 / 0	68 / 0	0 / 0	0 / 0	74 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 17	-84.3	-84.3	0.12 (1)	10.00
B- H	-414 / 0	-84.3	-84.3	0.07 (1)	6.25
H- C	-357 / 0	-84.3	-84.3	0.10 (1)	6.25
C- J	-357 / 0	-84.3	-84.3	0.10 (1)	6.25
J- D	-414 / 0	-84.3	-84.3	0.07 (1)	6.25
D- E	0 / 17	-84.3	-84.3	0.12 (1)	10.00
B- G	0 / 307	-28.0	-28.0	0.09 (1)	10.00
G- K	0 / 307	-28.0	-28.0	0.11 (1)	10.00
K- F	0 / 307	-28.0	-28.0	0.11 (1)	10.00
F- L	0 / 307	-28.0	-28.0	0.11 (1)	10.00
L- I	0 / 307	-28.0	-28.0	0.11 (1)	10.00
I- D	0 / 307	-28.0	-28.0	0.09 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	0 / 165	0.04 (2)
G- H	-33 / 114	0.00 (1)
I- J	-33 / 114	0.00 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	3-3-0	-2	-2	---	BACK	VERT	TOTAL
F	3-0-12	---	---	---	BACK	VERT	TOTAL
K	1-0-12	---	---	---	BACK	VERT	TOTAL
L	5-0-12	---	---	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.12 (A-B:1) , BC=0.11 (F-G:1) , WB=0.04 (C-F:2) , SSI=0.10 (C-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (D) (INPUT = 0.90)
JSI METAL= 0.10 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 41920-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285349	G93	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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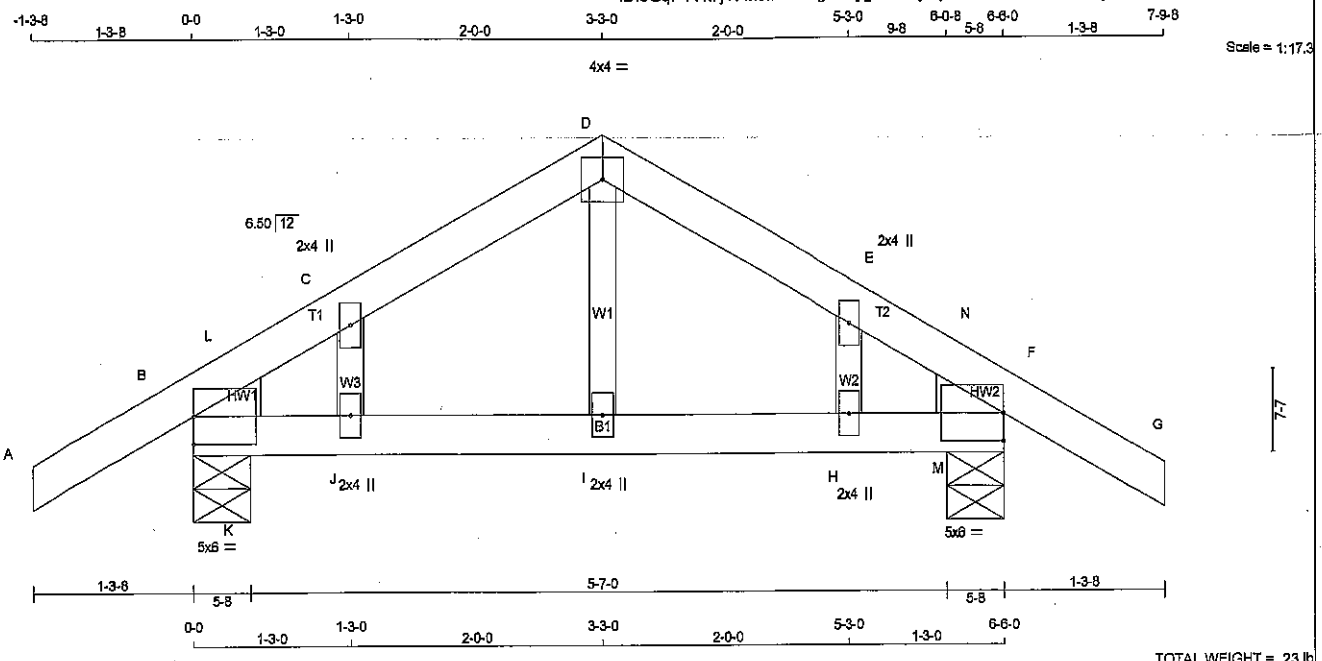

TOTAL WEIGHT = 23 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-I	MT20	5.0	6.0	2.50	
C	TMW+w	MT20	2.0	4.0		
D	TTW-p	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TMBH1-I	MT20	5.0	6.0	2.50	
H, I, J						
H	BMW+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
B	480	0	480	0	0	5-8	5-8	2x4 L
F	480	0	480	0	0	5-8	5-8	2x4 R

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	377	236 / 0	68 / 0	0 / 0	0 / 0	73 / 0	0 / 0
F	377	236 / 0	68 / 0	0 / 0	0 / 0	73 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 17	-84.3	-84.3	0.11 (1)	10.00	I- D	0 / 151	0.03 (2)
B- L	-525 / 0	-84.3	-84.3	0.11 (1)	6.25	H- E	-81 / 26	0.01 (1)
L- C	-322 / 0	-84.3	-84.3	0.05 (1)	6.25	J- C	-81 / 26	0.01 (1)
C- D	-298 / 0	-84.3	-84.3	0.06 (1)	6.25	K- L	0 / 225	0.00 (1)
D- E	-298 / 0	-84.3	-84.3	0.06 (1)	6.25	M- N	0 / 227	0.00 (1)
E- N	-322 / 0	-84.3	-84.3	0.05 (1)	6.25			
N- F	-527 / 0	-84.3	-84.3	0.11 (1)	6.25			
F- G	0 / 17	-84.3	-84.3	0.11 (1)	10.00			
B- K	0 / 267	-28.0	-28.0	0.11 (1)	10.00			
K- J	0 / 267	-28.0	-28.0	0.11 (1)	10.00			
J- I	0 / 261	-28.0	-28.0	0.08 (1)	10.00			
I- H	0 / 261	-28.0	-28.0	0.08 (1)	10.00			
H- M	0 / 267	-28.0	-28.0	0.11 (1)	10.00			
M- F	0 / 267	-28.0	-28.0	0.11 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.11/1.00 (F-N:1) , BC=0.11/1.00 (H-M:1) , WB=0.03/1.00 (D-I:2) , SSI=0.10/1.00 (F-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.66 (F) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)

DWG NO. TAM21186-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285348	P6	6	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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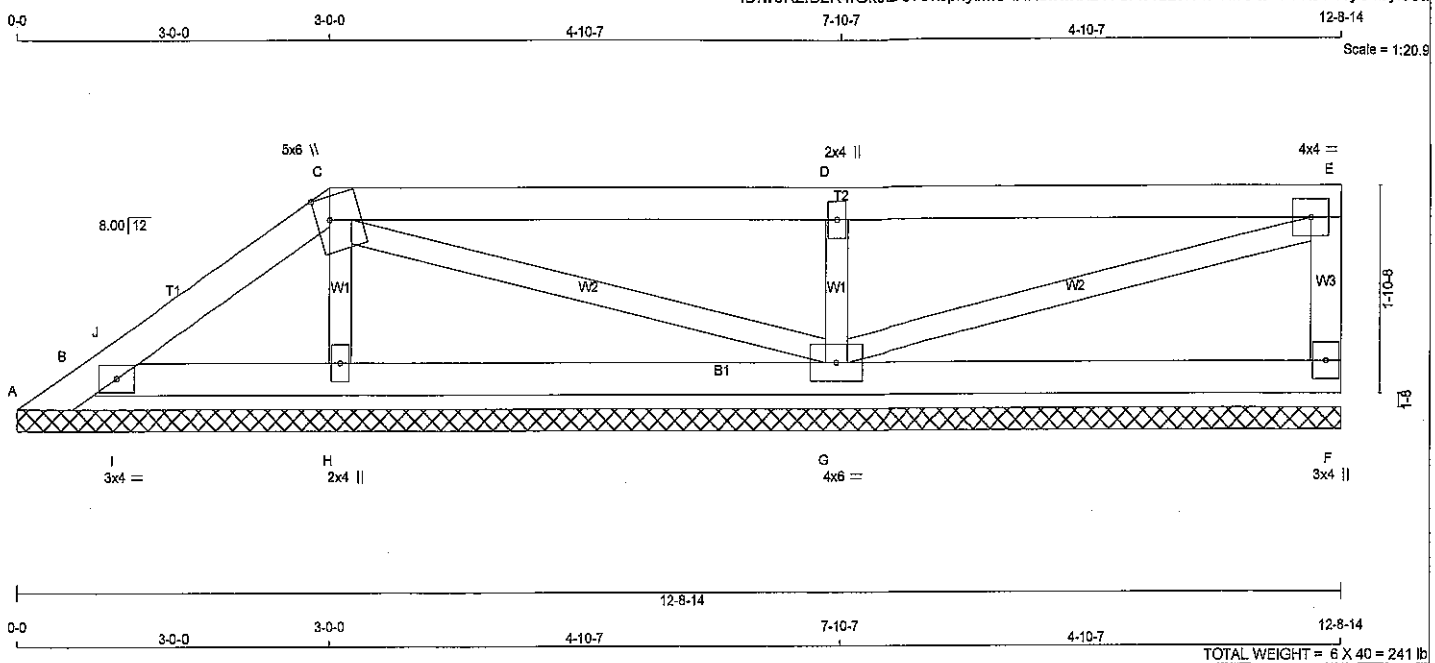

TOTAL WEIGHT = 6 X 40 = 241 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

BEARING NOTE: GAP BETWEEN INSIDE OF TOP CHORD BEARING AND FIRST DIAGONAL OR VERTICAL WEB SHALL NOT EXCEED 0.5 INCHES.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW+m	MT20	5.0	8.0	2.50	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW1-t	MT20	4.0	6.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
A	-12	0	3	0	-12	12-8-14 (11-11/16-14	
F	209	0	209	0	0	12-8-14 (11-11/16-14	
B	226	0	226	0	0	12-8-14 (11-11/16-14	
H	311	0	311	0	0	12-8-14 (11-11/16-14	
G	677	0	677	0	0	12-8-14 (11-11/16-14	

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): F , H , G

PROVIDE ANCHORAGE AT BEARING JOINT A FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	-9	0 / -9	0 / -1	0 / 0	0 / 0	2 / 0	0 / 0
F	173	94 / 0	40 / 0	0 / 0	0 / 0	36 / 0	0 / 0
B	188	122 / 0	16 / 0	0 / 0	0 / 0	30 / 0	0 / 0
H	271	125 / 0	78 / 0	0 / 0	0 / 0	67 / 0	0 / 0
G	550	315 / 0	119 / 0	0 / 0	0 / 0	116 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, F, B, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 23	-101.8	-101.8	0.05 (1)	10.00	H- C	-196 / 0	0.03 (1)	
B- J	-86 / 0	-84.3	-84.3	0.04 (1)	6.25	C- G	-23 / 0	0.01 (1)	
J- C	-45 / 0	-84.3	-84.3	0.04 (1)	6.25	G- D	-512 / 0	0.07 (1)	
C- D	0 / 2	-84.3	-84.3	0.34 (1)	10.00	G- E	-2 / 0	0.00 (1)	
D- E	0 / 2	-84.3	-84.3	0.34 (1)	10.00	I- J	-62 / 6	0.00 (1)	
F- E	-155 / 0	0.0	0.0	0.02 (1)	7.81				
B- I	0 / 33	-28.0	-28.0	0.03 (1)	10.00				
I- H	0 / 33	-28.0	-28.0	0.08 (2)	10.00				
H- G	0 / 20	-28.0	-28.0	0.17 (2)	10.00				
G- F	0 / 0	-28.0	-28.0	0.17 (3)	10.00				

LICENSED PROFESSIONAL ENGINEER
8/24/17
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 41923-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.34 (C-D:1) , BC=0.17 (G-H:2) , WB=0.07 (D-G:1) , SSI=0.20 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (D) (INPUT = 0.90)
JSI METAL= 0.09 (D) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285351	P8	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.210 S May 18 2018 MiTek Industries, Inc. Tue Jun 26 09:42:39 2018 Page 1
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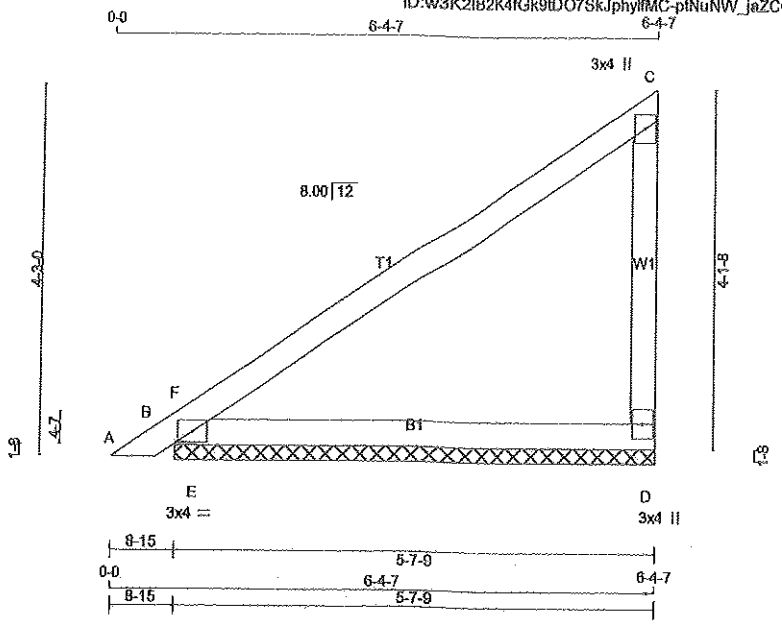

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	TOP
C-D	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
B-D	1	12	TOP

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMB1-I	MT20	3.0	4.0	
C	TMV+p	MT20	3.0	4.0	
D	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
D	316	0	316	0	0	5-7-9	5-7-9
B	372	0	372	0	0	5-7-9	5-7-9

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	259	144 / 0	59 / 0	0 / 0	0 / 0	56 / 0	0 / 0
B	297	178 / 0	59 / 0	0 / 0	0 / 0	60 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, B

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 14	-84.3	-84.3	0.01 (1)		10.00
B-F	-168 / 142	-84.3	-84.3	0.04 (1)		6.25
F-C	-7 / 6	-84.3	-84.3	0.12 (1)		10.00
D-C	-211 / 0	0.0	0.0	0.02 (1)		7.81
B-E	0 / 0	-28.0	-28.0	0.07 (1)		10.00
E-D	0 / 0	-28.0	-28.0	0.09 (1)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E-F	-473 / 257	0.00 (1)

TOTAL WEIGHT = 3 X 19 = 57 lb (M)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.12/1.00 (C-F:1) , BC=0.09/1.00 (D-E:1) , WB=0.00/1.00 (E-F:1) , SSI=0.14/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	622	2284	1650

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.08 (B) (INPUT = 0.90)
JSI METAL= 0.02 (B) (INPUT = 1.00)

DWG NO. TAM 19006044
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285351	P8C	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.210 S May 18 2016 MiTek Industries, Inc. Tue Jun 26 09:42:40 2018 Page 1
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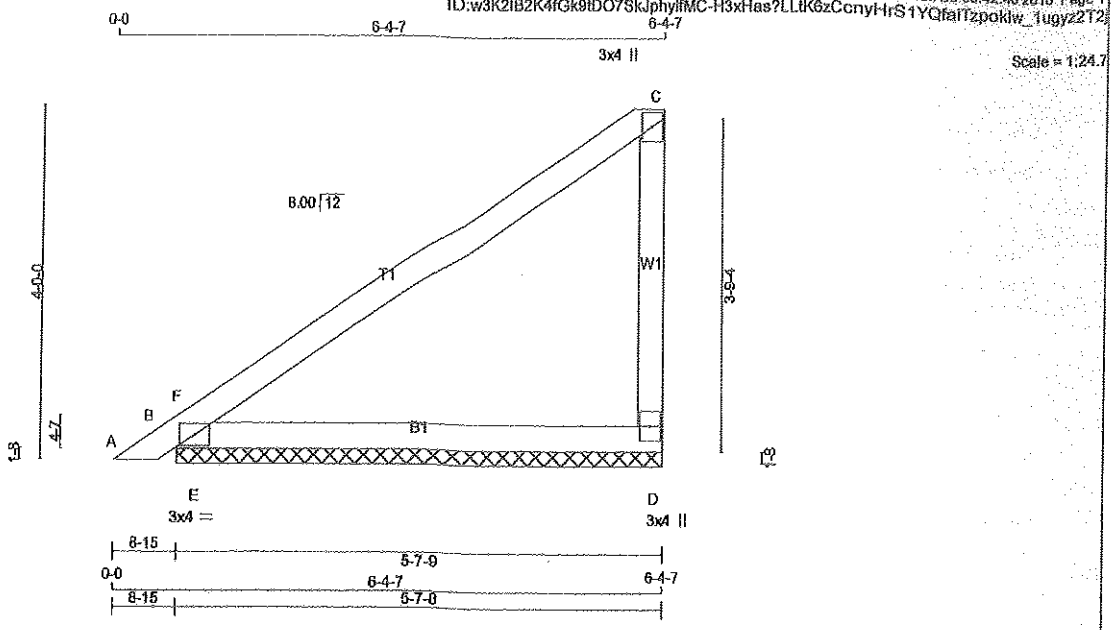


TOTAL WEIGHT = 19 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TMV+p	MT20	3.0	4.0		
D	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	316	0	316	0	0	5-7-9 (5-7-8)	5-7-9
B	372	0	372	0	0	5-7-9 (5-7-8)	5-7-9

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	259	144 / 0	59 / 0	0 / 0	0 / 0	56 / 0	0 / 0
B	297	178 / 0	59 / 0	0 / 0	0 / 0	60 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, B

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 14	-84.3	-84.3	0.03 (1)	10.00	
B-F	-169 / 145	-84.3	-84.3	0.12 (1)	6.25	
F-C	-7 / 8	-84.3	-84.3	0.38 (1)	10.00	
D-C	-211 / 0	0.0	0.0	0.05 (1)	7.81	
B-E	0 / 0	-28.0	-28.0	0.22 (1)	10.00	
E-D	0 / 0	-28.0	-28.0	0.27 (1)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E-F	-477 / 259	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.38/1.00 (C-F:1) , BC=0.27/1.00 (D-E:1) , WB=0.00/1.00 (E-F:1) , SSI=0.41/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

DWG NO. TAM 180645
STRUCTURAL
COMPONENT ONLY

TECHNICAL BULLETIN

LUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

MATERIAL: 18 gauge

FINISH: G90 galvanized

DESIGN:

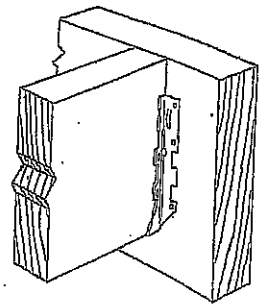
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

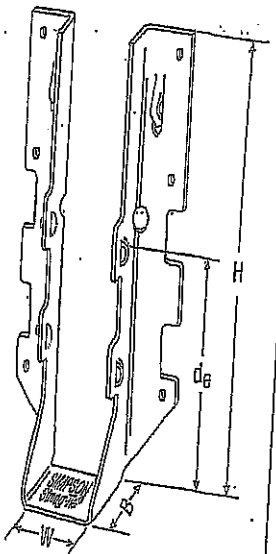
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

- These hangers cannot be modified.



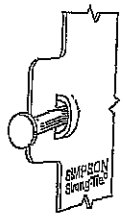
Typical LUS Installation



LUS28

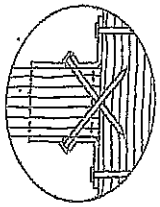
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
								D.Fir-L		S-P-F	
		W	H	B	d_g[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LUS24	18	1⅜	3⅛	1¾	11⅝	4-10d	2-10d	710	1630	645	1165
LUS24-2	18	3⅛	3⅛	2	11⅝	4-16d	2-16d	885	2020	590	1495
LUS26	18	1⅜	4¾	1¾	3⅝	4-10d	4-10d	1420	2170	1290	1630
LUS26-2	18	3⅛	4⅞	2	4	4-16d	4-16d	1720	2595	1545	1920
LUS26-3	18	4⅝	4⅞	2	3¼	4-16d	4-16d	1720	2595	1545	2840
LUS28	18	1⅜	6⅝	1¾	3¾	6-10d	4-10d	1420	2520	1290	1790
LUS28-2	18	3⅛	7	2	4	6-16d	4-16d	1720	3825	1545	2575
LUS28-3	18	4⅝	6¼	2	3¼	6-16d	4-16d	1720	3825	1545	2575
LUS210	18	1⅜	7⅞	1¾	3⅝	8-10d	4-10d	1420	2785	1290	2210
LUS210-2	18	3⅛	9	2	6	8-16d	6-16d	2680	4500	2320	3165
LUS210-3	18	4⅝	8⅞	2	5¼	8-16d	6-16d	2680	3845	2320	2975

1. d_g is the distance from the seat of the hanger to the highest joist nail.

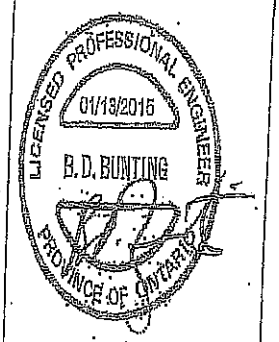


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This Technical Bulletin is effective until December 31, 2016, and reflects information available at January 1, 2016. This information is updated periodically and should not be relied upon after December 31, 2016. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

T-SPECLUS16 1/16 exp. 12/16

800-999-5099

strongtie.com

TECHNICAL BULLETIN

HUS/LJS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: See table

FINISH: G90 galvanized

DESIGN:

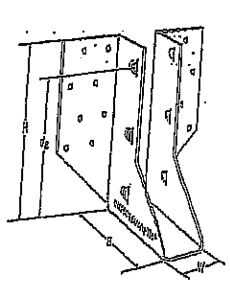
- Factored resistances are in accordance with CSA 086-14
- Uplift resistances have been increased 15% No further increase is permitted
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

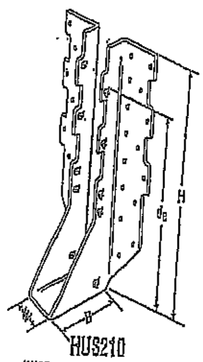
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

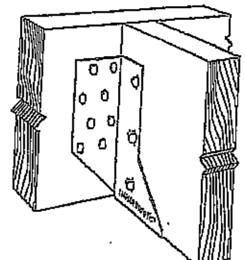
- See current catalogue for options



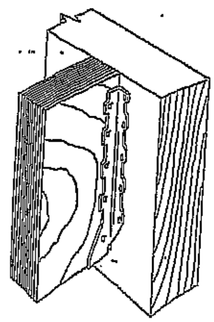
LJS26DS



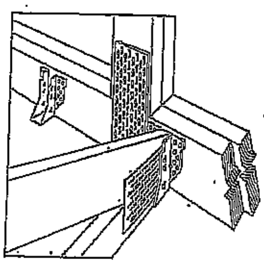
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS Installation



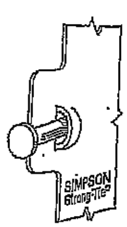
Typical HUS Installation



Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

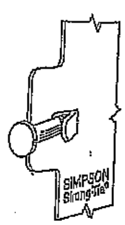
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LJS26DS	18	1$\frac{9}{16}$	5	3½	4⅝	16-16d	6-16d	2065	4265	1460	4115
HUS26	16	1⅝	5⅜	3	3$\frac{15}{16}$	14-16d	6-16d	2705	4940	2065	3875
HUS28	16	1⅝	7$\frac{3}{32}$	3	6$\frac{3}{32}$	22-16d	8-16d	3605	5365	2675	4345
HUS210	16	1⅝	9$\frac{3}{32}$	3	7$\frac{31}{32}$	30-16d	10-16d	4505	5795	4010	4740
HUS1.81/10	16	1$\frac{13}{16}$	9	3	8	30-16d	10-16d	4505	6450	4010	5200

1. d_e is the distance from the seat of the hanger to the highest joist nail.

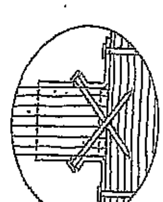


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

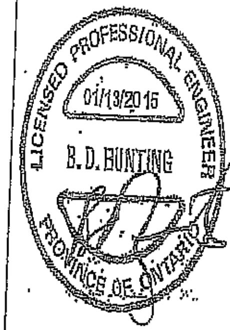
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.

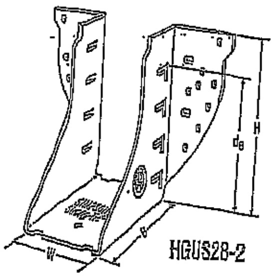


TECHNICAL BULLETIN

SIMPSON Strong-Tie ®

HGUS – Double Shear Joist Hangers

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.



MATERIAL: 12 gauge

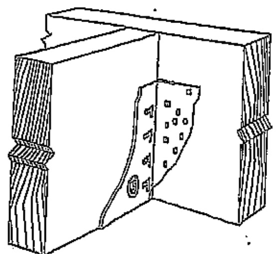
FINISH: G90 galvanized

DESIGN:

- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

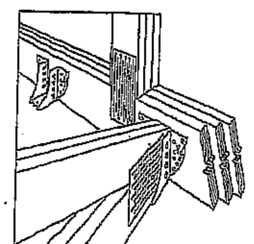
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications



Typical HGUS Installation

OPTIONS:

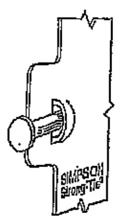
- See current catalogue for options.



Typical HGUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)

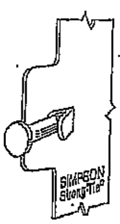
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
HGUS26	12	1⅝	5⅜	5	4 5/32	20-16d	8-16d	2685	6625	2685	5700
HGUS26-2	12	3 5/16	5 7/16	4	4⅛	20-16d	8-16d	4385	8950	3100	6355
HGUS26-3	12	4 15/16	5½	4	4⅛	20-16d	8-16d	4885	8950	3100	6365
HGUS26-4	12	6 9/16	5 7/16	4	4⅛	20-16d	8-16d	4385	8950	3100	6355
HGUS28	12	1⅝	7⅛	5	6⅛	36-16d	12-16d	3310	7675	3100	6900
HGUS28-2	12	3 5/16	7 9/16	4	6⅛	36-16d	12-16d	6070	12980	4310	9215
HGUS28-3	12	4 15/16	7¼	4	6⅛	36-16d	12-16d	6070	12980	4310	9215
HGUS28-4	12	6 9/16	7 9/16	4	6⅛	36-16d	12-16d	6070	12980	4910	9215
HGU210-2	12	3 5/16	9 9/16	4	8⅛	46-16d	16-16d	6840	14645	4855	10400
HGUS210-3	12	4 15/16	9¼	4	8⅛	46-16d	16-16d	6840	14645	4855	10400
HGUS210-4	12	6 9/16	9 9/16	4	8⅛	46-16d	16-16d	6840	14645	4856	10400
HGUS212-4	12	6 9/16	10⅝	4	10⅛	56-16d	20-16d	7640	14995	5426	10645
HGUS214-4	12	6 9/16	12⅝	4	11⅛	66-16d	22-16d	10130	18400	7195	11645

1. d_e is the distance from the seat of the hanger to the highest joist nail.

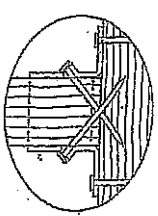


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

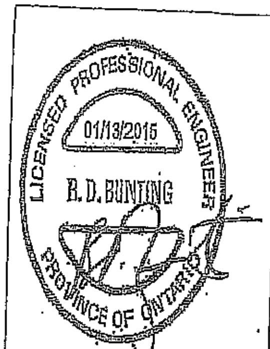
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.

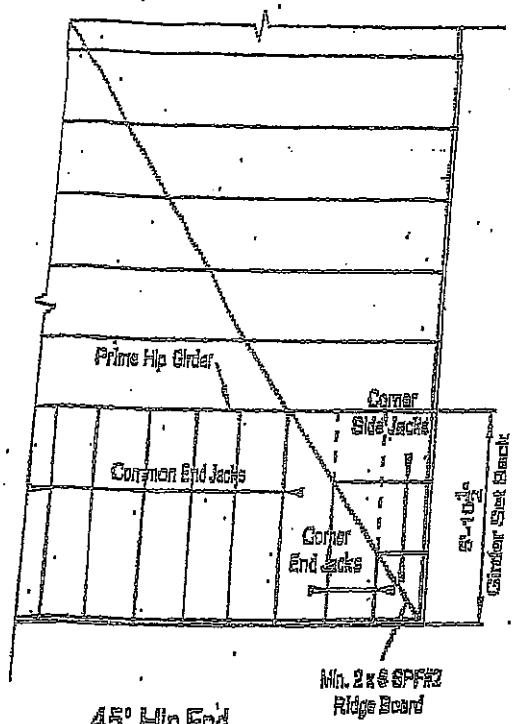


Double Shear Nailing Top View.



MICRO CITY
ENGINEERING SERVICES INC.

TEL: (619) 287 - 2242



45° Hip End

LUMBER SPECIFICATION

TOP CHORD : 2x4 SPF#2

BOTTOM CHORD : 2x4 SPF#2

WEBS : 2x3 SPF#2

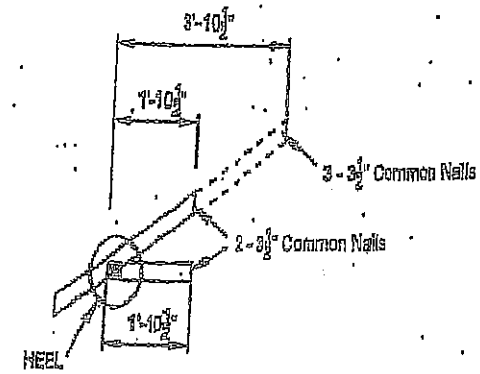
UNLESS OTHERWISE SHOWN

DESIGN LOAD:

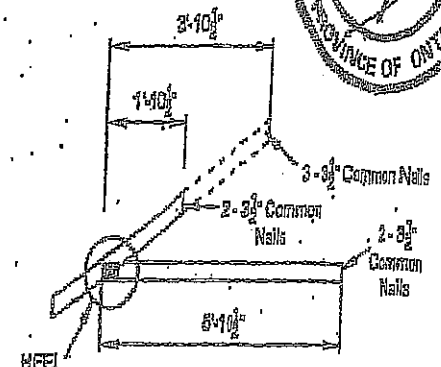
TOP CHORD LIVE LOAD	:	39.4 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	10.5 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F
TOTAL LOAD		59.9 P.S.F.

DWG NO TIM2415.14
STRUCTURAL COMPONENT ONLY

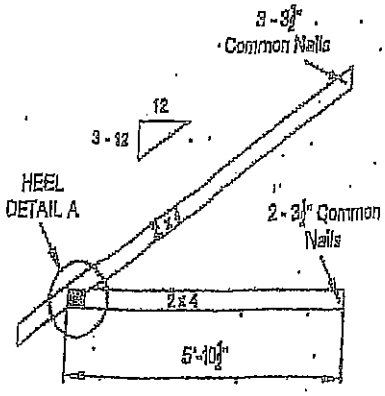
LICENSED PROFESSIONAL ENGINEER
1-20-14
S. KATSOULAKOS
PROVINCE OF ONTARIO



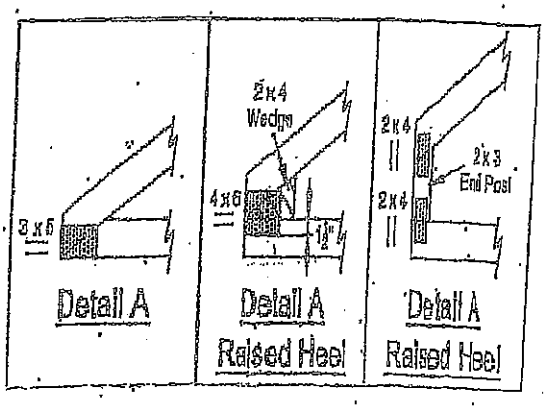
Corner Side Jacks



Corner End Jacks



Common End Jacks

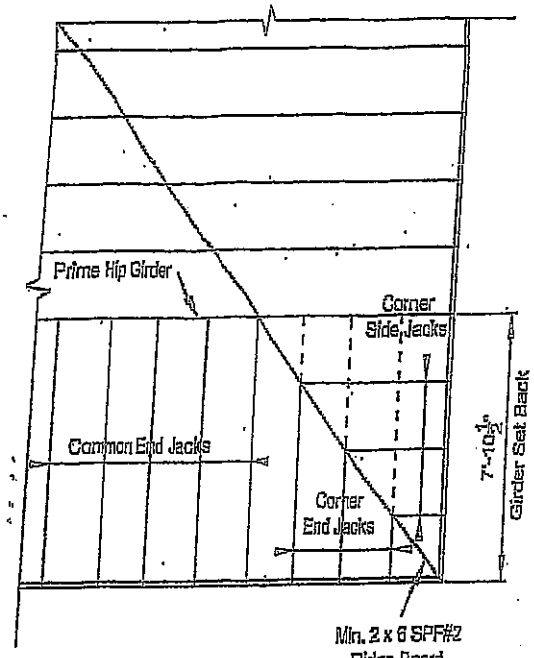


NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)

MICRO CITY
ENGINEERING SERVICES INC.
TEL: (519) 287 - 2242
R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

LUMBER SPECIFICATION

TOP CHORD : 2x4 SPF#2
BOTTOM CHORD : 2x4 SPF#2
WEBS : 2x3 SPF#2
UNLESS OTHERWISE SHOWN



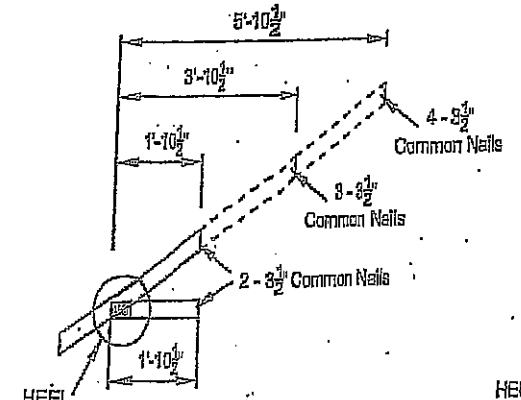
45° Hip End

DESIGN LOAD:

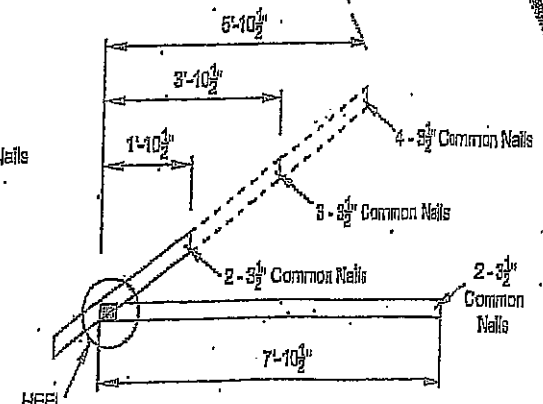
TOP CHORD LIVE LOAD	: 34.8 P.S.F.
TOP CHORD DEAD LOAD	: 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	: 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	: 7.0 P.S.F.
TOTAL LOAD	: 44.8 P.S.F.

DWG NO TAN 3503 IA
STRUCTURAL
COMPONENT ONLY

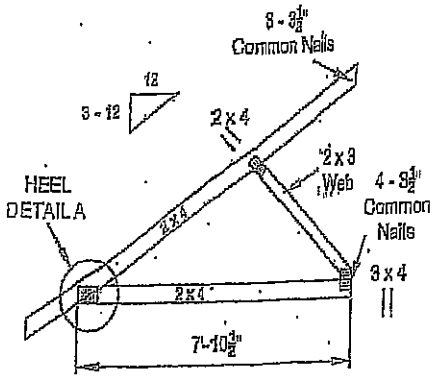
1-2014
S. KATSOULAKOS
LICENSED PROFESSIONAL ENGINEER
PROVINCE OF ONTARIO



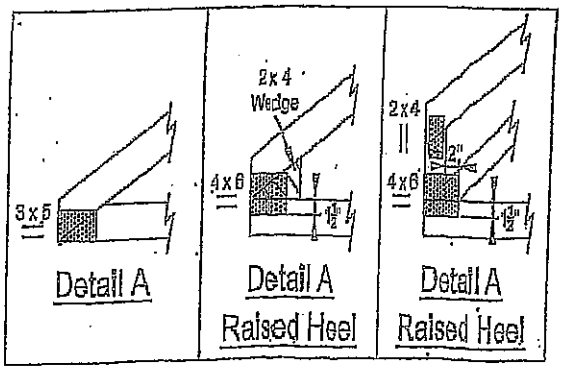
Corner Side Jacks



Corner End Jacks



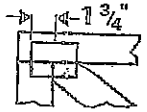
Common End Jacks



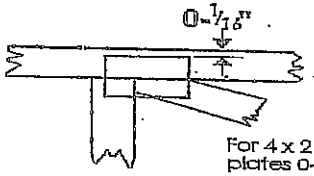
NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

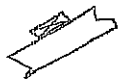
*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4

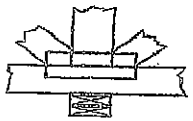
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

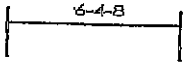
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses

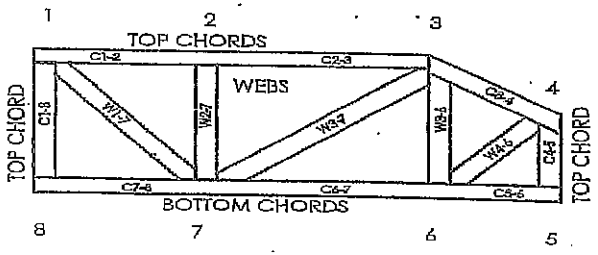
DSB-89: *Design Standard for Bracing.*

BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

MiTek®

POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MII-7473C rev. 10-'08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

Micro City Engineering Services Inc.
(BCIN: 26064; FIRM BCIN: 29991)
RR #1, Po Box 61
Glencoe, Ontario
N0L 1M0
(519) 287 - 2242; Fax: (519) 287 - 5750 (Call)

DWG NO [illegible] 7414
STRUCTURAL
COMPONENT ONLY

Responsibilities:

Micro City Engineering Services is responsible for the design of trusses as individual components.

It is the responsibilities of others to ascertain that the design loads utilized on this (these) drawing(s) meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdiction over such decisions.

All dimensions are to be verified by the owner, contractor, architect, or other authority having input over such decisions prior to truss component manufacture. At no time shall Micro City Engineering Services Inc. or its employees be responsible for dimension errors.

Micro City Engineering Services Inc. bears no responsibility for the erection of any truss components. Persons erecting truss components are cautioned to seek professional advice regarding temporary and permanent bracing systems and to be totally familiar with all aspects of truss erection prior to proceeding on any truss component erection job. Any bracing shown on Micro City Engineering Services Inc. or Tamarack Roof Trusses Inc. sealed or unsealed truss component drawings is specified for the single truss component in question and is identified as an integral part of the design for that particular truss component but is not meant to represent the only required bracing for that particular truss component when installed as a component in a series of truss components in a roof truss system.

~~It is the truss manufacturer's responsibility to ensure that trusses are manufactured in accordance with Micro City Engineering~~ Services Inc. specifications outlined below:

SPECIFICATIONS:

Truss components sealed by Micro City Engineering Services Inc. must conform to the relevant sections of the current Building Code of Ontario and Canada (Part 4 or Part 9) or the current Farm Building Code of Canada in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the Truss Plate Institute of Canada (TPIC). All unit lumber and nailing stresses identified on truss component design drawings and/or used in the design of individual truss components shall conform to the current CSA Wood Design standard identified in the current Building Code and TPIC Design Standards.

The lumber used to manufacture any truss component is to conform to the specified size and grade identified on the truss drawing.

The lumber used in the manufacture of any truss component is not to exceed 19% during its service use unless specifically noted on the truss drawing.

The lumber used in the manufacture of any truss component is not to be treated with any chemicals during its service life unless specifically noted on the truss drawing.

Connector plates shall be applied to both faces of the truss component at each joint and shall be positioned exactly as specified.

The top chord of any truss component is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the sealed truss component drawing but not exceeding 24" o/c (Part 9 design) and not exceeding 48" o/c (Part 4 or Agricultural design).

When a truss component is to be installed with no rigid ceiling attached directly to the bottom chord, then the bottom chord is to be laterally braced at intervals not exceeding 3m (or 10'-0").

All sealed or unsealed truss component drawings provided by Micro City Engineering Services Inc. Or Tamarack Roof Trusses Inc. should be read in conjunction with the following:

Warning-Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473C rev 10-'08 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer - not the truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component Safety Information available from the Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA, 22314.