



Job Track: **43954**
Layout ID: **282696**
Plan Log: **92461**

Builder / Location: **GREENPARK HOMES / WATERDOWN**
Project: **RUSSEL GARDENS PHASE 2**
Date: **8/25/2017** Designer: **colins V.E**

Model / Elevator: **ROSEWOOD 4 / 1**
THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

MD979

T - 170375

ASPHALT SHINGLES
12" FINISHED O.H.
5" CLADDING ALLOWANCE
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. 2012 ROOF RAFTERS THAT CROSS OVER OR MEET TRUSSES TO BE 2x4 SPF #2 24" O.C. WITH A VERT POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DIST. BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'



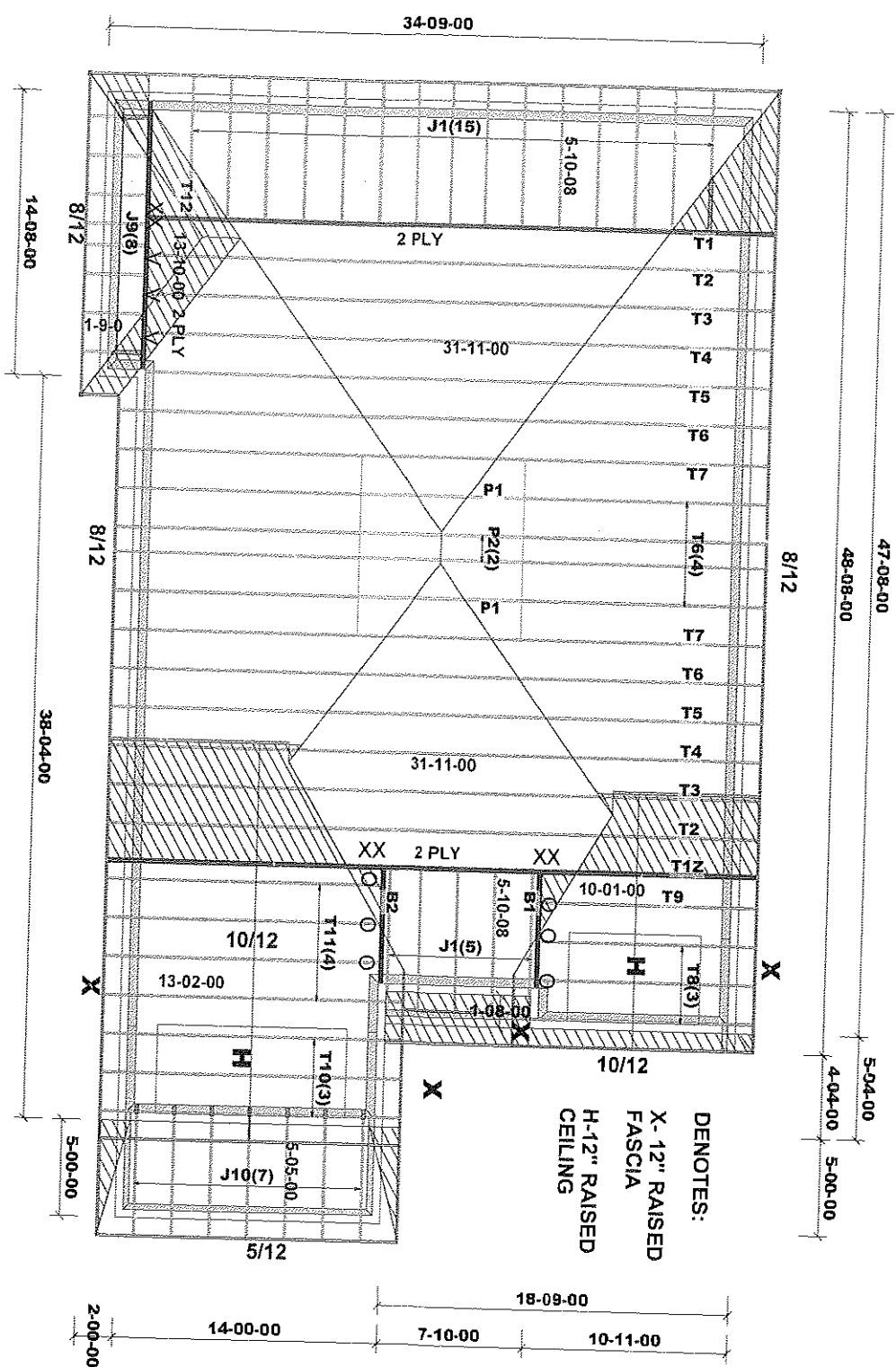
DENOTES:
CONVENTIONAL FRAMING

ALL ROOF SLOPES ARE 6/12 UNLESS OTHERWISE NOTED

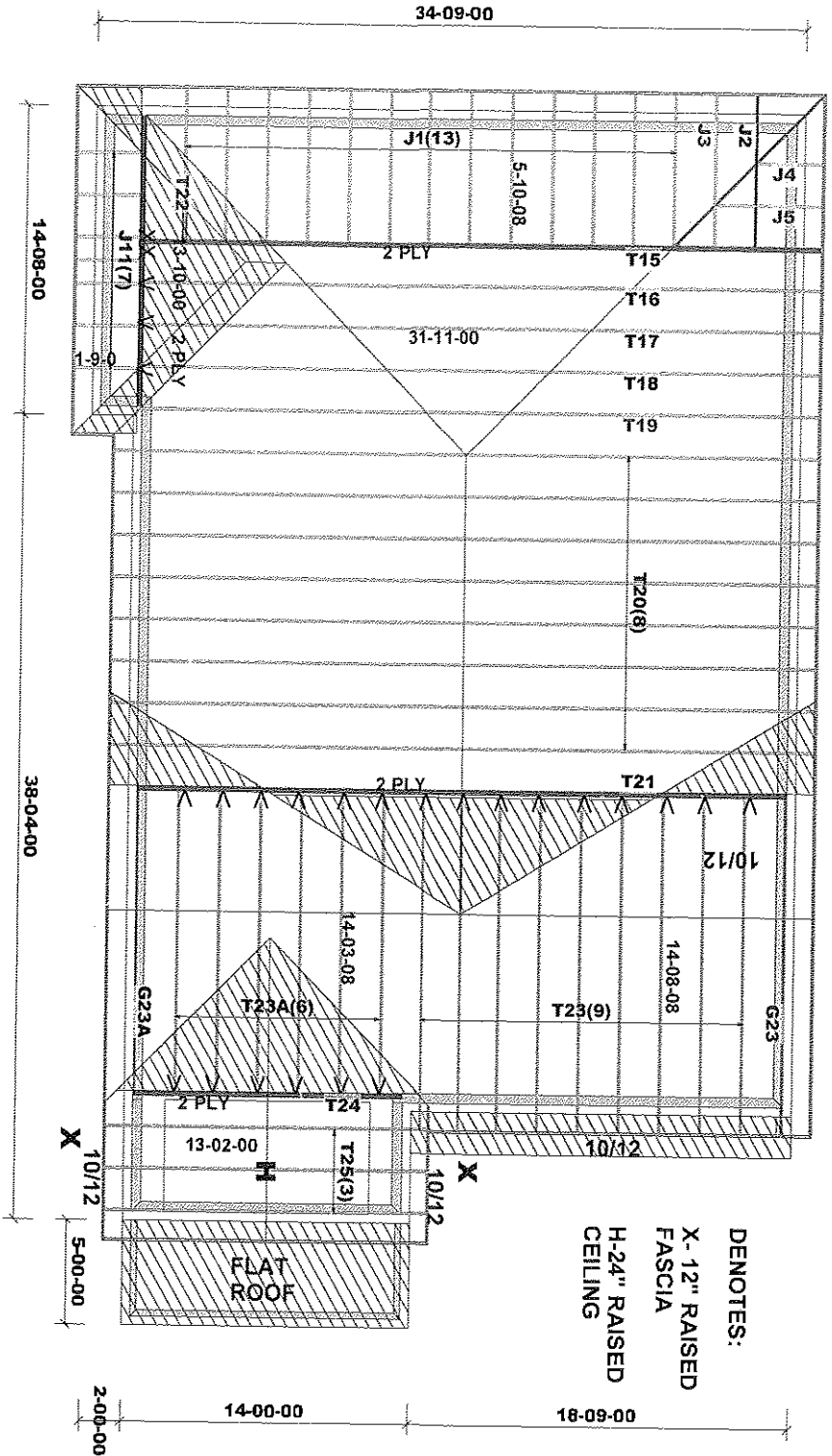
BEAMS:
ALL BEAMS (B): 2 - 2X10 SPF #2

DESIGN CONFORMS WITH OBC 2012 OCCUPANCY: RESIDENTIAL / PART: 9 GROUND SNOW LOAD $S_s=31.3\text{psf}$ $S_r=8.4\text{psf}$ TCCL (3psf) BCLL (10.5psf) BCDL (7psf)

HARDWARE:
HGUS26-2 -(XX)
LUS26DS -(V)
LUS24 -(O)



47'-08"-00
48'-08"-00
5'-04"-00
4'-04"-00
5'-00"-00



DENOTES:
X - 12" RAISED
FASCIA
H-24" RAISED
CEILING

ASPHALT SHINGLES
12" FINISHED O.H.
5" CLADDING ALLOWANCE
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD
HEEL: R.T.M.C.

ALL CONV. FRAMING TO CONFORM
WITH PART 9 OF O.B.C. 2012 ROOF
RAFTERS THAT CROSS OVER OR
MEET TRUSSES TO BE 2X4 SPF #2 24"
O.C. WITH A VERT POST TO THE TRUSS
UNDERNEATH AT EACH CROSS PT.
VERT. POST LONGER THAN 6'
TO HAVE LATERAL BRACING SO
THAT THE DIST. BETWEEN END PT.
& BETWEEN ROWS OF BRACING
DOES NOT EXCEED 6'

DESIGN CONFORMS WITH OBC 2012
OCCUPANCY: RESIDENTIAL | PART. 9
GROUND SNOW LOAD $S_s=31.3$ psf $S_r=8.4$ psf
DESIGN LOADS
TCDL (3psf)
BCLL (10.5psf)
BCDL (7psf)

HARDWARE:
HGUS26-2 -(XX)
LJS26DS -(V)

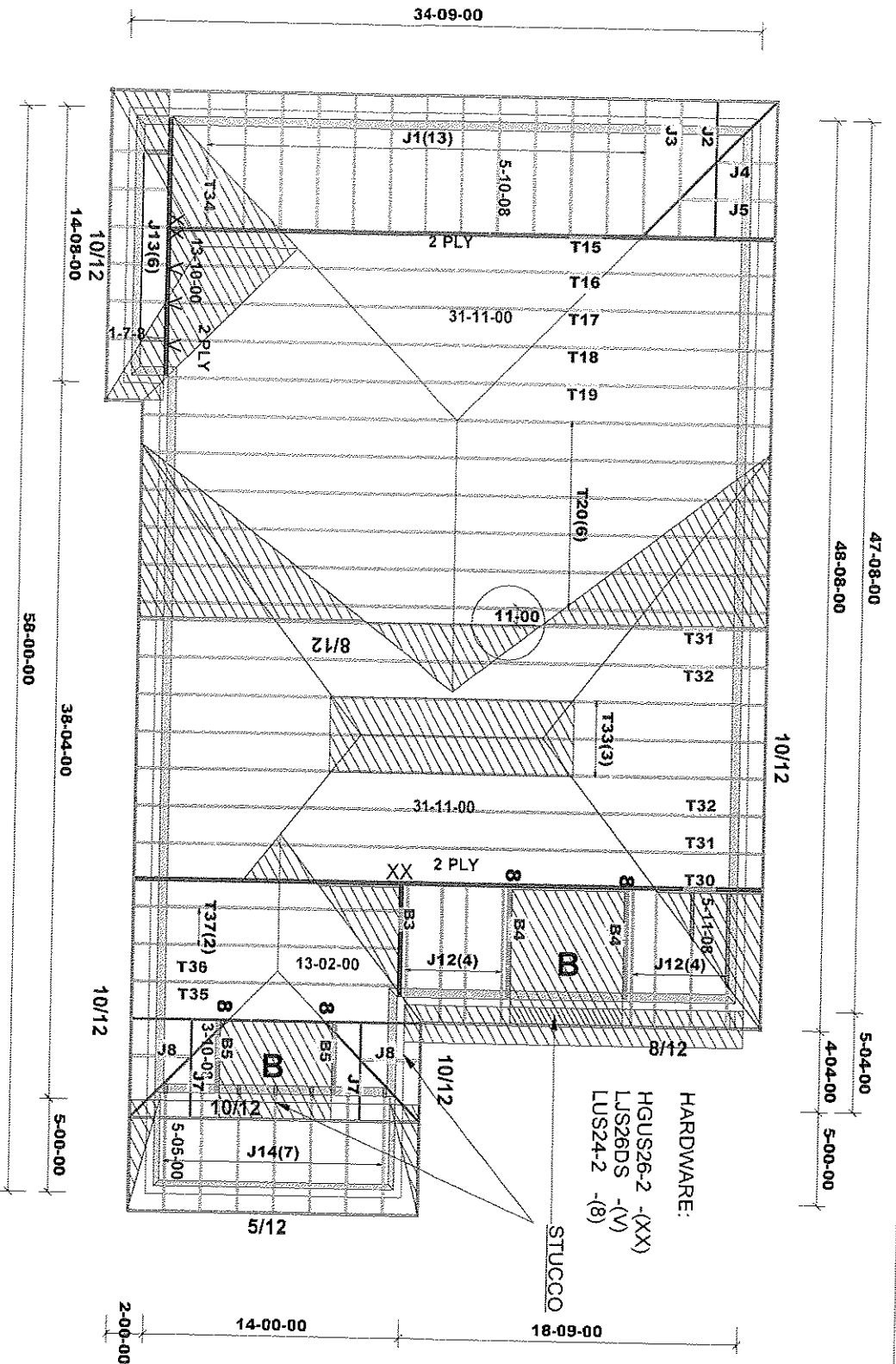


DENOTES:
CONVENTIONAL
FRAMING

ALL ROOF SLOPES ARE 6/12
UNLESS OTHERWISE NOTED

10/9/19

Job Track: 43954		Builder / Location: GREENPARK HOMES / WATERDOWN		Model / Elevation: ROSEWOOD 4 / 2	
Layout ID: 282697		Project: RUSSEL GARDENS PHASE 2			
Plan Log: 92461		Date: 8/25/2017 Designer: colins v e			
THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE. Mitek ver 7.5.0					



ASPHALT SHINGLES
12" FINISHED O.H.
5" CLADDING ALLOWANCE
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

T - 170375

ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. 2012 ROOF RAFTERS THAT CROSS OVER OR MEET TRUSSES TO BE 2X4 SPF #2 24" O.C. WITH A VERT POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DIST. BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'



DENOTES:
CONVENTIONAL
FRAMING

ALL ROOF SLOPES ARE 6/12
UNLESS OTHERWISE NOTED

DESIGN CONFORMS WITH OBC 2012 OCCUPANCY: RESIDENTIAL / PART: 9 GROUND SNOW LOAD $S_s=31.3$ psf $S_r=8.4$ psf
DESIGN LOADS
TCDL (3psf)
BCLL (10.5psf)
BCDL (7psf)

B: BARREL VAULT CEILING
(C.R.F.)

BEAMS:
B3: 2 - 2X10 SPF #2
B4, B5: 2-2X8 SPF #2

HARDWARE:
HGU26-2 -(XX)
LJS26DS -(W)
LUS24-2 -(8)

STUCCO

Builder / Location:

GREENPARK HOMES / WATERDOWN

Model / Elevation:

ROSEWOOD 4 / 3

Job Track: 43954

Layout ID: 282698

Plan Log: 92461

Project:

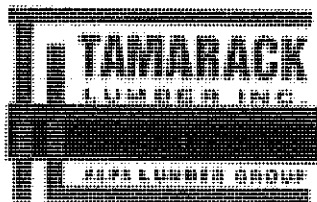
RUSSEL GARDENS PHASE 2

Date:

8/25/2017

Designer: colin/s.v.e

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.



Delivery Shiplist

DATE	08/25/17
SALES REP	Mario

JOB TRACK:43954 LAYOUT ID: 282696 LOCATION: WATERDOWN
 BUILDER: GREENPARK - RUSSELL GARDENS PH 2 SUB-BUILDER:
 MODEL: ROSEWOOD 4 ELEVATION: 1

ROOF TRUSSES

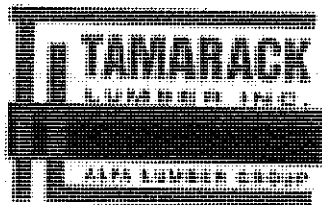
ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY PLY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER TOP BOT	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	1 2 Ply	T1 HIP GIRDER	8.00 0.00	31-11-00	04-01-04	2 X 6 2 X 6	01-03-08 01-03-08	01-04-13 01-04-13	364.66 218.68		
	1 2 Ply	T1Z HIP GIRDER	8.00 0.00	31-11-00	04-01-04	2 X 6 2 X 6	01-03-08 01-03-08	01-04-13 01-04-13	364.66 218.68		
	2	T2 HIP	8.00 0.00	31-11-00	05-01-04	2 X 4 2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	252.00 161.66		
	2	T3 HIP	8.00 0.00	31-11-00	06-01-04	2 X 4 2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	271.88 172.00		
	2	T4 HIP	8.00 0.00	31-11-00	07-01-04	2 X 4 2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	267.76 170.34		
	2	T5 HIP	8.00 0.00	31-11-00	08-01-04	2 X 4 2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	287.78 182.00		
	6	T6 HIP	8.00 0.00	31-11-00	09-01-04	2 X 4 2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	862.50 544.02		
	2	T7 HIP	8.00 0.00	31-11-00	10-01-04	2 X 4 2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	293.10 185.34		
	1 2 Ply	T12 HIP GIRDER	6.00 0.00	13-10-00	02-06-13	2 X 4 2 X 6	00-00-00 00-00-00	01-02-00 01-02-00	131.88 86.66		
	3	T10 ROOF	10.00 0.00	13-02-00	08-01-08	2 X 4 2 X 4	01-03-08 01-03-08	02-07-11 02-07-11	214.08 142.50		
	4	T11 COMMON	10.00 0.00	13-02-00	08-01-08	2 X 4 2 X 4	01-03-08 01-03-08	02-07-11 02-07-11	274.12 176.00		
	3	T8 ROOF	10.00 0.00	10-01-00	06-10-02	2 X 4 2 X 4	01-03-08 01-03-08	02-07-11 02-07-11	179.70 123.99		
	1	T9 COMMON	10.00 0.00	10-01-00	06-10-02	2 X 4 2 X 4	01-03-08 01-03-08	02-07-11 02-07-11	49.32 32.83		
	2	P1 PIGGYBACK	8.00 0.00	07-03-13	01-10-08	2 X 4 2 X 4	00-00-00 00-00-00	00-04-07 00-04-07	48.16 32.34		
	2	P2 PIGGYBACK	8.00 0.00	07-03-13	02-09-12	2 X 4 2 X 4	00-00-00 00-00-00	00-04-07 00-04-07	44.36 29.66		
	20	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4 2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	335.80 213.40		
	8	J9 JACK-OPEN	8.00 0.00	01-09-00	02-06-13	2 X 4 2 X 4	01-03-08 00-00-00	01-04-13 02-06-13	59.76 37.36		
	7	J10 JACK-OPEN	5.00 0.00	05-05-00	02-07-02	2 X 4 2 X 4	01-03-08 00-00-00	00-04-01 00-06-01	102.20 65.31		

TOTAL # TRUSS= 72.00

TOTAL BFT OF ALL TRUSSES=

2792.77 BFT. TOTAL WEIGHT OF ALL TRUSSES= 4403.72 LBS.



Delivery Shiplist

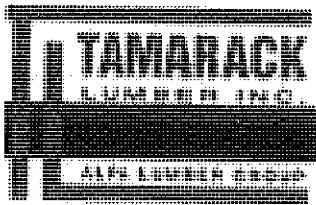
DATE	08/25/17
SALES REP	Mario

JOB TRACK: 43954	LAYOUT ID: 282696	LOCATION: WATERDOWN
BUILDER: GREENPARK - RUSSELL GARDENS PH 2	SUB-BUILDER:	
MODEL: ROSEWOOD 4	ELEVATION: 1	

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	HGUS26-2	
3	Hangers	LJS26DS	
6	Hangers	LUS24	

TOTAL # ITEMS= 12.00



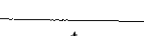
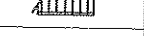
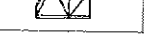
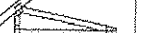
Delivery Shiplist

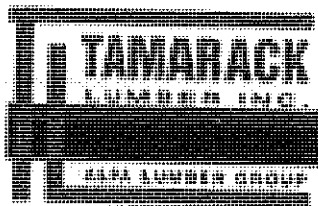
DATE	08/25/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 282697 LOCATION: WATERDOWN
 BUILDER: GREENPARK - RUSSELL GARDENS PH 2 SUB-BUILDER:
 MODEL: ROSEWOOD 4 ELEVATION: 2

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)												
PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	T15 HIP GIRDER	6.00	31-11-00	04-01-04	2 X 4	2 X 6	01-03-08	01-02-00	323.04		
	2 Ply		0.00					01-03-08	01-02-00	198.68		
	1	T16 HIP	6.00	31-11-00	05-01-04	2 X 4	2 X 4	01-03-08	01-02-00	127.46		
			0.00					01-03-08	01-02-00	79.83		
	1	T17 HIP	6.00	31-11-00	06-01-04	2 X 4	2 X 4	01-03-08	01-02-00	130.07		
			0.00					01-03-08	01-02-00	81.34		
	1	T18 HIP	6.00	31-11-00	07-01-04	2 X 4	2 X 4	01-03-08	01-02-00	131.32		
			0.00					01-03-08	01-02-00	81.33		
	1	T19 HIP	6.00	31-11-00	08-01-04	2 X 4	2 X 4	01-03-08	01-02-00	132.23		
			0.00					01-03-08	01-02-00	83.17		
	8	T20 COMMON	6.00	31-11-00	09-01-12	2 X 4	2 X 4	01-05-00	01-02-00	1044.16		
			0.00					01-05-00	01-02-00	644.00		
	1	T21 COMMON	6.00	31-11-00	09-01-12	2 X 4	2 X 6	00-00-00	01-02-00	329.12		
	2 Ply		0.00					00-00-00	01-02-00	200.00		
	1	T22 HIP GIRDER	6.00	13-10-00	02-00-08	2 X 4	2 X 6	00-00-00	01-02-00	125.88		
	2 Ply		0.00					00-00-00	01-02-00	84.00		
	9	T23 COMMON	10.00	14-08-08	09-01-13	2 X 4	2 X 4	01-03-08	01-07-11	627.57		
			0.00					00-00-00	04-04-13	393.03		
	1	G23 GABLE	10.00	14-08-08	09-01-13	2 X 4	2 X 4	01-03-08	01-07-11	78.09		
			0.00					00-00-00	04-04-13	49.17		
	6	T23A COMMON	10.00	14-03-08	09-01-13	2 X 4	2 X 4	00-00-00	01-11-14	401.34		
			0.00					00-00-00	04-04-13	253.98		
	1	G23A GABLE	10.00	14-03-08	09-01-13	2 X 4	2 X 4	00-00-00	01-11-14	75.47		
			0.00					00-00-00	04-04-13	47.83		
	1	T24 COMMON	10.00	13-02-00	08-01-08	2 X 4	2 X 6	00-00-00	02-07-11	169.36		
	2 Ply		0.00					00-00-00	02-07-11	106.66		
	3	T25 ROOF	10.00	13-02-00	08-01-08	2 X 4	2 X 4	01-03-08	02-07-11	215.22		
			0.00					01-03-08	02-07-11	144.99		
	13	J1 JACK-OPEN	6.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08	01-02-00	218.27		
			0.00					00-00-00	04-01-04	138.71		
	1	J2 JACK-OPEN	6.00	05-10-08	02-00-12	2 X 4	2 X 4	01-03-08	01-02-00	16.16		
			0.00					-04-01-01	03-09-05	10.33		
	1	J3 JACK-OPEN	6.00	05-10-08	03-00-12	2 X 4	2 X 4	01-03-08	01-02-00	18.71		
			0.00					-02-01-01	03-09-05	11.67		
	1	J4 JACK-OPEN	6.00	01-10-08	02-00-12	2 X 4	2 X 4	01-03-08	01-02-00	8.43		
			0.00					-00-01-01	01-09-05	5.67		



Delivery Shiplist

Page 2 of 2

DATE	08/25/17
SALES REP	Mario

JOB TRACK: 43954	LAYOUT ID: 282697	LOCATION: WATERDOWN
BUILDER: GREENPARK - RUSSELL GARDENS PH 2	SUB-BUILDER:	
MODEL: ROSEWOOD 4	ELEVATION: 2	

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	J5 JACK-OPEN	6.00 0.00	01-10-08	03-00-12	2 X 4	2 X 4	01-03-08 01-10-15	01-02-00 00-04-11	10.98 7.00		
	7	J11 JACK-OPEN	6.00 0.00	01-09-00	02-00-08	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 02-00-08	47.81 32.69		

TOTAL # TRUSS= 64.00

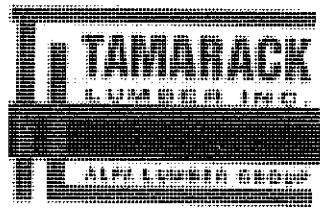
TOTAL BFT OF ALL TRUSSES=

2654.08 BFT. TOTAL WEIGHT OF ALL TRUSSES= 4230.69 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	HGUS26-2	
24	Hangers	LJS26DS	

TOTAL # ITEMS= 25.00



Delivery Shiplist

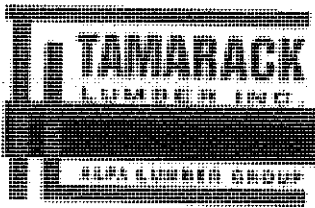
DATE	08/25/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 282698 LOCATION: WATERDOWN
 BUILDER: GREENPARK - RUSSELL GARDENS PH 2 SUB-BUILDER:
 MODEL: ROSEWOOD 4 ELEVATION: 3

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY PLY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER TOP BOT	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	1	T15	6.00	31-11-00	04-01-04	2 X 4 2 X 6	01-03-08	01-02-00	323.04		
	2 Ply	HIP GIRDER	0.00						198.68		
	1	T16	6.00	31-11-00	05-01-04	2 X 4 2 X 4	01-03-08	01-02-00	127.46		
		HIP	0.00						79.83		
	1	T17	6.00	31-11-00	06-01-04	2 X 4 2 X 4	01-03-08	01-02-00	130.07		
		HIP	0.00						81.34		
	1	T18	6.00	31-11-00	07-01-04	2 X 4 2 X 4	01-03-08	01-02-00	131.31		
		HIP	0.00						81.33		
	1	T19	6.00	31-11-00	08-01-04	2 X 4 2 X 4	01-03-08	01-02-00	132.23		
		HIP	0.00						83.17		
	6	T20	6.00	31-11-00	09-01-12	2 X 4 2 X 4	01-05-00	01-02-00	783.12		
		COMMON	0.00						483.00		
	1	T30	10.00	31-11-00	05-04-08	2 X 6 2 X 6	01-03-08	01-07-11	393.62		
	2 Ply	HIP GIRDER	0.00						232.00		
	2	T31	10.00	31-11-00	06-08-08	2 X 4 2 X 4	01-03-08	01-07-11	274.10		
		HIP	0.00						171.00		
	2	T32	10.00	31-11-00	08-00-08	2 X 4 2 X 4	01-03-08	01-07-11	303.78		
		HIP	0.00						191.34		
	3	T33	10.00	31-11-00	09-04-08	2 X 4 2 X 4	01-03-08	01-07-11	468.03		
		HIP	0.00						290.49		
	1	T34	6.00	13-10-00	03-01-03	2 X 4 2 X 6	00-00-00	01-02-00	125.28		
	2 Ply	HIP GIRDER	0.00						82.66		
	1	T35	10.00	13-02-00	04-10-07	2 X 4 2 X 4	01-03-08	01-07-11	61.26		
		HIP GIRDER	0.00						40.00		
	1	T36	10.00	13-02-00	06-06-07	2 X 4 2 X 4	01-03-08	01-07-11	65.15		
		HIP	0.00						42.17		
	2	T37	10.00	13-02-00	07-01-08	2 X 4 2 X 4	01-03-08	01-07-11	125.54		
		COMMON	0.00						80.34		
	13	J1	6.00	05-10-08	04-01-04	2 X 4 2 X 4	01-03-08	01-02-00	218.27		
		JACK-OPEN	0.00						138.71		
	1	J2	6.00	05-10-08	02-00-12	2 X 4 2 X 4	01-03-08	01-02-00	16.16		
		JACK-OPEN	0.00						10.33		
	1	J3	6.00	05-10-08	03-00-12	2 X 4 2 X 4	01-03-08	01-02-00	18.71		
		JACK-OPEN	0.00						11.67		
	1	J4	6.00	01-10-08	02-00-12	2 X 4 2 X 4	01-03-08	01-02-00	8.43		
		JACK-OPEN	0.00						5.67		



Delivery Shiplist

DATE	08/25/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 282698 LOCATION: WATERDOWN
 BUILDER: GREENPARK - RUSSELL GARDENS PH 2 SUB-BUILDER:
 MODEL: ROSEWOOD 4 ELEVATION: 3

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	J5 JACK-OPEN	6.00 0.00	01-10-08	03-00-12	2 X 4	2 X 4	01-03-08 01-10-15	01-02-00 00-04-11	10.98 7.00		
	2	J7 JACK-OPEN	10.00 0.00	03-10-08	03-01-09	2 X 4	2 X 4	01-03-08 -02-01-01	01-07-11 04-05-14	27.46 18.66		
	2	J8 JACK-OPEN	10.00 0.00	01-10-08	03-01-09	2 X 4	2 X 4	01-03-08 -00-01-01	01-07-11 02-09-14	19.98 14.00		
	8	J12 JACK-OPEN	8.00 0.00	05-11-08	05-04-08	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-04-08	144.32 85.36		
	6	J13 JACK-OPEN	10.00 0.00	01-09-00	03-01-03	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 03-01-03	49.02 36.00		
	7	J14 JACK-OPEN	5.00 0.00	05-05-00	02-07-02	2 X 4	2 X 4	01-03-08 00-00-00	00-04-01 00-06-01	102.20 65.31		

TOTAL # TRUSS= 69.00

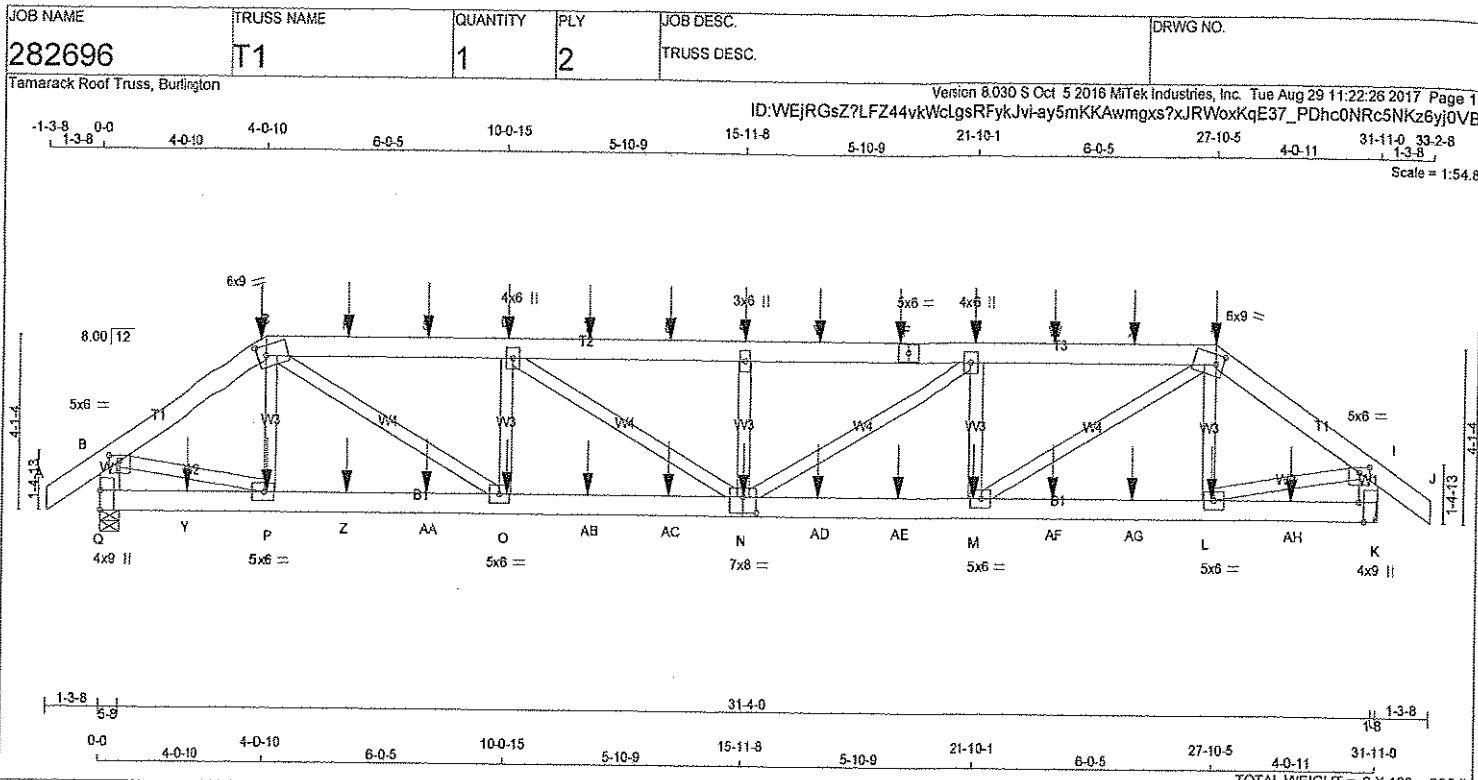
TOTAL BFT OF ALL TRUSSES=

2530.06 BFT. TOTAL WEIGHT OF ALL TRUSSES= 4059.52 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
4	Hangers	LUS24-2	
3	Hangers	LJS26DS	

TOTAL # ITEMS= 9.00



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x6	No.2	SPF
C - F	2x6	No.2	SPF
F - H	2x6	No.2	SPF
H - J	2x6	No.2	SPF
Q - B	2x6	No.2	SPF
K - I	2x6	No.2	SPF
Q - N	2x6	1650F 1.5E	SPF
N - K	2x6	1650F 1.5E	SPF

ALL WEBS 2x4 DRY No.2 SPF
EXCEPT

DRY, SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	2	12
C - F	2	12
F - H	2	12
H - J	2	12
Q - B	2	12
K - I	2	12
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
Q - N	2	12
N - K	2	12
WEBS : (0.122"x3") SPIRAL NAILS		
2x4	1	6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	5.0	6.0	1.50	3.00
C	TTWW-m	MT20	6.0	9.0	2.75	2.25
D	TMWW-t	MT20	4.0	6.0		
E	TMWW-w	MT20	3.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMWW-t	MT20	4.0	6.0		
H	TTWW-m	MT20	6.0	9.0	2.75	2.25
I	TMWW-p	MT20	5.0	6.0	1.50	3.00
K	BMV-t	MT20	4.0	9.0	Edge	1.50
L	M. O. P.					
L	BMWW-t	MT20	5.0	6.0		
N	BSWW-t	MT20	7.0	8.0	4.50	4.00
Q	BMV-t	MT20	4.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	2873	2873	0	0
Q	2873	2873	0	0
K	2874	2874	0	0

HANGER BY OTHERS
MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
Q	2376	1289 / 0	580 / 0	0 / 0	0 / 0	527 / 0	0 / 0
K	2377	1290 / 0	580 / 0	0 / 0	0 / 0	527 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.52 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS1 (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 33	-84.3 -84.3	0.03 (1)	10.00	P-C	-325 / 138 0.03 (1)	
B-C	-3446 / 0	-84.3 -84.3	0.08 (1)	5.94	C-O	0 / 3228 0.29 (1)	
C-R	-5813 / 0	-84.3 -84.3	0.22 (1)	4.79	O-D	-1394 / 0 0.12 (1)	
R-S	-5813 / 0	-84.3 -84.3	0.22 (1)	4.79	D-N	0 / 978 0.09 (1)	
S-D	-5813 / 0	-84.3 -84.3	0.22 (1)	4.79	N-E	-744 / 0 0.06 (1)	
D-T	-6445 / 0	-84.3 -84.3	0.23 (1)	4.52	N-G	0 / 975 0.09 (1)	
T-U	-6445 / 0	-84.3 -84.3	0.23 (1)	4.52	M-G	-1395 / 0 0.12 (1)	
U-E	-6445 / 0	-84.3 -84.3	0.23 (1)	4.52	M-H	0 / 3230 0.29 (1)	
E-V	-6445 / 0	-84.3 -84.3	0.23 (1)	4.52	L-H	-325 / 138 0.03 (1)	
V-F	-6445 / 0	-84.3 -84.3	0.23 (1)	4.52	B-P	0 / 2924 0.26 (1)	
F-G	-6445 / 0	-84.3 -84.3	0.23 (1)	4.52	L-I	0 / 2926 0.26 (1)	
G-W	-5816 / 0	-84.3 -84.3	0.22 (1)	4.79			
W-X	-5816 / 0	-84.3 -84.3	0.22 (1)	4.79			
X-H	-5816 / 0	-84.3 -84.3	0.22 (1)	4.79			
H-I	-3448 / 0	-84.3 -84.3	0.08 (1)	5.94			
I-J	0 / 33	-84.3 -84.3	0.03 (1)	10.00			
Q-B	-2823 / 0	0.0 0.0	0.10 (1)	7.81			
K-I	-2825 / 0	0.0 0.0	0.10 (1)	7.81			

Q-Y	0 / 0	-28.0	-28.0 0.05 (2)	10.00
Y-P	0 / 0	-28.0	-28.0 0.05 (2)	10.00
P-Z	0 / 2847	-28.0	-28.0 0.15 (1)	10.00
Z-AA	0 / 2847	-28.0	-28.0 0.15 (1)	10.00
AA-O	0 / 2847	-28.0	-28.0 0.15 (1)	10.00
O-AB	0 / 5813	-28.0	-28.0 0.27 (1)	10.00
AB-AC	0 / 5813	-28.0	-28.0 0.27 (1)	10.00
AC-N	0 / 5813	-28.0	-28.0 0.27 (1)	10.00
N-AD	0 / 5816	-28.0	-28.0 0.27 (1)	10.00
AD-AE	0 / 5816	-28.0	-28.0 0.27 (1)	10.00
AE-M	0 / 5816	-28.0	-28.0 0.27 (1)	10.00
M-AF	0 / 2848	-28.0	-28.0 0.15 (1)	10.00
AF-AG	0 / 2848	-28.0	-28.0 0.15 (1)	10.00
AG-L	0 / 2848	-28.0	-28.0 0.15 (1)	10.00
L-AH	0 / 0	-28.0	-28.0 0.05 (2)	10.00
AH-K	0 / 0	-28.0	-28.0 0.05 (2)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(5% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.06")
CALCULATED VERT. DEFL.(LL)= L/999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (1.06")
CALCULATED VERT. DEFL.(TL)= L/999 (0.23")

CSI: TC=0.23 (D-E:1), BC=0.27 (M-N:1), WB=0.29 (H-I:1), SSI=0.14 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354 1657 822 2284 1856	

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (H) (INPUT = 0.90)
JSI METAL= 0.39 (N) (INPUT = 1.00)



DWG NO. TAM 43634-17
STRUCTURAL
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CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
282696	T1	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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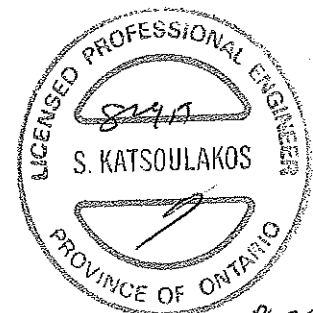
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HANGERS NOTES

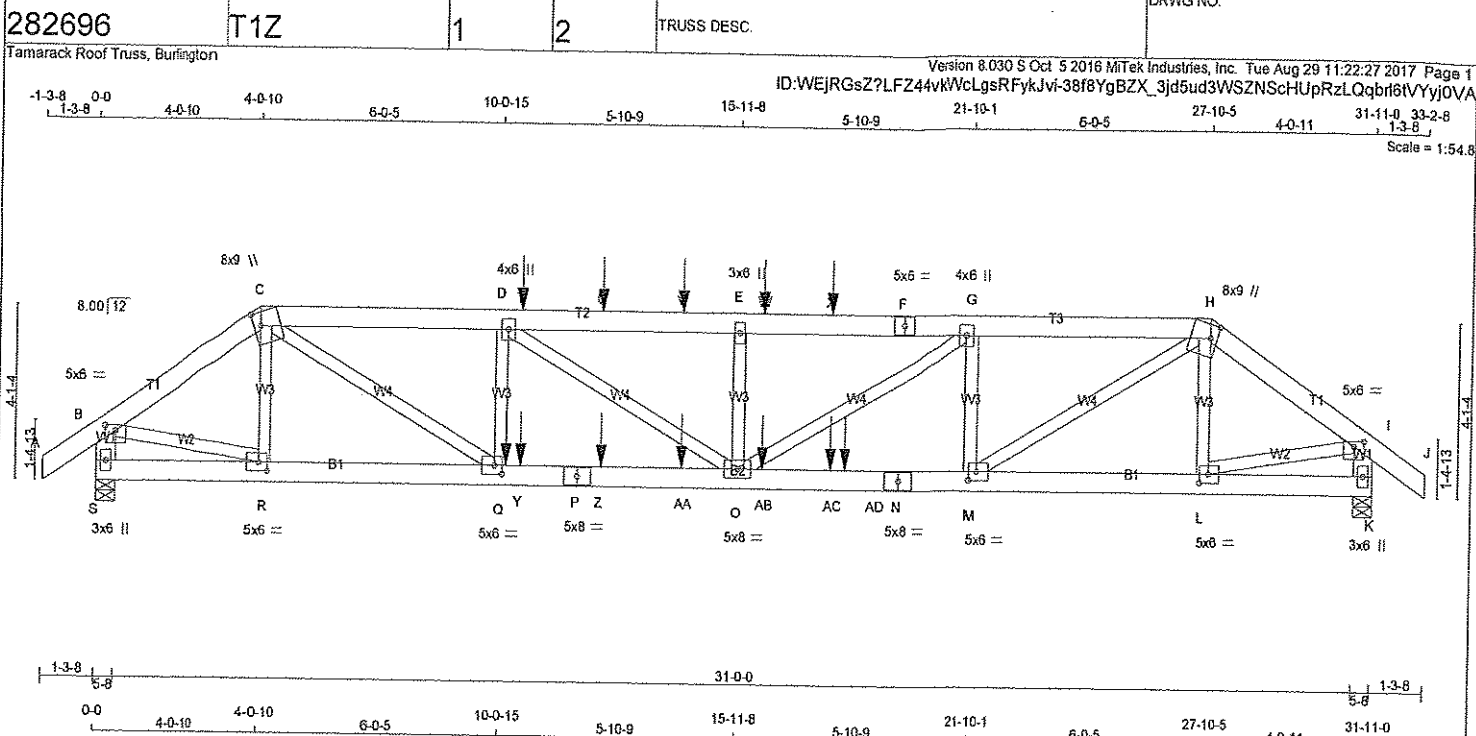
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 104.9 lbs FACTORED DOWN AT 4-0-11, 101.4 lbs FACTORED DOWN AT 6-1-6, 101.4 lbs FACTORED DOWN AT 8-1-6, 101.4 lbs FACTORED DOWN AT 10-1-6, 101.4 lbs FACTORED DOWN AT 12-1-6, 101.4 lbs FACTORED DOWN AT 14-1-6, 101.4 lbs FACTORED DOWN AT 15-11-9, 101.4 lbs FACTORED DOWN AT 17-9-9, 101.4 lbs FACTORED DOWN AT 19-9-9, 101.4 lbs FACTORED DOWN AT 21-9-9, 101.4 lbs FACTORED DOWN AT 23-9-9, AND 101.4 lbs FACTORED DOWN AT 25-9-9, AND 104.9 lbs FACTORED DOWN AT 27-10-6 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 2-1-6, 69.9 lbs FACTORED DOWN AT 4-1-6, 69.9 lbs FACTORED DOWN AT 6-1-6, 69.9 lbs FACTORED DOWN AT 8-1-6, 69.9 lbs FACTORED DOWN AT 10-1-6, 69.9 lbs FACTORED DOWN AT 12-1-6, 69.9 lbs FACTORED DOWN AT 14-1-6, 69.9 lbs FACTORED DOWN AT 15-11-9, 69.9 lbs FACTORED DOWN AT 17-9-9, 69.9 lbs FACTORED DOWN AT 19-9-9, 69.9 lbs FACTORED DOWN AT 21-9-9, 69.9 lbs FACTORED DOWN AT 23-9-9, 69.9 lbs FACTORED DOWN AT 25-9-9, AND 69.9 lbs FACTORED DOWN AT 27-9-9, AND 69.9 lbs FACTORED DOWN AT 29-9-9 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	4-0-11	-105	-105	---	FRONT	VERT	TOTAL
D	10-1-8	-101	-101	---	FRONT	VERT	TOTAL
E	15-11-9	-101	-101	---	FRONT	VERT	TOTAL
F	19-9-9	-101	-101	---	FRONT	VERT	TOTAL
G	21-9-9	-101	-101	---	FRONT	VERT	TOTAL
H	27-10-6	-105	-105	---	FRONT	VERT	TOTAL
L	27-9-9	-40	-70	---	FRONT	VERT	TOTAL
M	21-9-9	-40	-70	---	FRONT	VERT	TOTAL
N	15-11-9	-40	-70	---	FRONT	VERT	TOTAL
O	10-1-6	-40	-70	---	FRONT	VERT	TOTAL
P	4-1-6	-40	-70	---	FRONT	VERT	TOTAL
R	6-1-6	-101	-101	---	FRONT	VERT	TOTAL
S	8-1-6	-101	-101	---	FRONT	VERT	TOTAL
T	12-1-6	-101	-101	---	FRONT	VERT	TOTAL
U	14-1-6	-101	-101	---	FRONT	VERT	TOTAL
V	17-9-9	-101	-101	---	FRONT	VERT	TOTAL
W	23-9-9	-101	-101	---	FRONT	VERT	TOTAL
X	25-9-9	-101	-101	---	FRONT	VERT	TOTAL
Y	2-1-6	-40	-70	---	FRONT	VERT	TOTAL
Z	6-1-6	-40	-70	---	FRONT	VERT	TOTAL
AA	8-1-6	-40	-70	---	FRONT	VERT	TOTAL
AB	12-1-6	-40	-70	---	FRONT	VERT	TOTAL
AC	14-1-6	-40	-70	---	FRONT	VERT	TOTAL
AD	17-9-9	-40	-70	---	FRONT	VERT	TOTAL
AE	19-9-9	-40	-70	---	FRONT	VERT	TOTAL
AF	23-9-9	-40	-70	---	FRONT	VERT	TOTAL
AG	25-9-9	-40	-70	---	FRONT	VERT	TOTAL
AH	29-9-9	-40	-70	---	FRONT	VERT	TOTAL



DWG NO. TAM 43634-17
STRUCTURAL
COMPONENT ONLY



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x6	DRY	No.2 SPF
C - F	2x6	DRY	No.2 SPF
F - H	2x6	DRY	No.2 SPF
H - J	2x6	DRY	No.2 SPF
S - B	2x6	DRY	No.2 SPF
K - I	2x6	DRY	No.2 SPF
S - P	2x6	DRY	1650F 1.5E SPF
P - N	2x6	DRY	1650F 1.5E SPF
N - K	2x6	DRY	1650F 1.5E SPF
ALL WEBS EXCEPT	2x4	DRY	No.2 SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	2	12 TOP
C - F	2	12 SIDE(0.0)
F - H	2	12 TOP
H - J	2	12 TOP
S - B	2	12 TOP
K - I	2	12 TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
S - P	2	12 SIDE(183.1)
P - N	2	12 SIDE(0.0)
N - K	2	12 TOP
WEBS : (0.122"x3") SPIRAL NAILS		
D - Q	1	6 SIDE(93.1)
2x4	1	6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	5.0	6.0	1.50	3.00
C	TTWW+m	MT20	6.0	9.0	3.75	2.00
D	TMWW+t	MT20	4.0	6.0		
E	TMWW+w	MT20	3.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMWW+H	MT20	4.0	6.0		
H	TTWW+m	MT20	8.0	9.0	3.75	2.00
I	TMWW-p	MT20	5.0	6.0	1.50	3.00
K	BMWW-t	MT20	5.0	6.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.75
M	BMWW-t	MT20	5.0	6.0	2.50	2.25
N	BS-t	MT20	5.0	8.0		
O	BMWWWW-t	MT20	5.0	8.0		
P	BS-t	MT20	5.0	8.0		
Q	BMWW-t	MT20	5.0	6.0	2.50	2.25
R	BMWW-t	MT20	5.0	6.0	2.50	2.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	UP
S	3679	0	3679	0
K	3489	0	3489	0

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	2957	1745 / 0	605 / 0	0 / 0	0 / 0	888 / 0	0 / 0
K	2807	1651 / 0	577 / 0	0 / 0	0 / 0	579 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.71 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)	
FR-TO				FR-TO			
A-B	0 / 33	-84.3	-84.3 0.03 (1)	10.00	R-C	-563 / 0	0.05 (1)
B-C	-4583 / 0	-84.3	-84.3 0.09 (1)	5.32	C-Q	0 / 5250	0.46 (1)
C-D	-8280 / 0	-84.3	-84.3 0.23 (1)	4.08	Q-D	-1652 / 0	0.14 (1)
D-T	-9827 / 0	-84.3	-84.3 0.31 (1)	3.71	D-O	0 / 1821	0.16 (1)
T-U	-9827 / 0	-84.3	-84.3 0.31 (1)	3.71	O-E	-774 / 0	0.07 (1)
U-V	-9827 / 0	-84.3	-84.3 0.31 (1)	3.71	O-G	0 / 2164	0.19 (1)
V-E	-9827 / 0	-84.3	-84.3 0.31 (1)	3.71	M-G	-1696 / 0	0.15 (1)
E-W	-9827 / 0	-84.3	-84.3 0.29 (1)	3.73	L-H	0 / 5247	0.48 (1)
W-X	-9827 / 0	-84.3	-84.3 0.29 (1)	3.73	H-H	-758 / 0	0.06 (1)
X-F	-9827 / 0	-84.3	-84.3 0.29 (1)	3.73	B-R	0 / 3989	0.34 (1)
F-G	-9827 / 0	-84.3	-84.3 0.29 (1)	3.73	L-I	0 / 3801	0.32 (1)
G-H	-7988 / 0	-84.3	-84.3 0.29 (1)	4.14			
H-I	-4244 / 0	-84.3	-84.3 0.09 (1)	5.48			
I-J	0 / 33	-84.3	-84.3 0.03 (1)	10.00			
S-B	-3659 / 0	0.0	0.0 0.13 (1)	7.35			
K-I	-3410 / 0	0.0	0.0 0.12 (1)	7.55			

JT	TYPE	PLATES	W	LEN	Y	X
S-R	0 / 0	-28.0	-28.0 0.04 (2)	10.00		
R-Q	0 / 3781	-28.0	-28.0 0.22 (1)	10.00		
Q-Y	0 / 8280	-28.0	-28.0 0.53 (1)	10.00		
Y-P	0 / 8280	-28.0	-28.0 0.53 (1)	10.00		
P-Z	0 / 8280	-28.0	-28.0 0.53 (1)	10.00		
Z-AA	0 / 8280	-28.0	-28.0 0.53 (1)	10.00		
AA-O	0 / 8280	-28.0	-28.0 0.53 (1)	10.00		
O-AB	0 / 7988	-28.0	-28.0 0.78 (1)	10.00		
AB-AC	0 / 7988	-28.0	-28.0 0.78 (1)	10.00		
AC-AD	0 / 7988	-28.0	-28.0 0.78 (1)	10.00		
AD-N	0 / 7988	-28.0	-28.0 0.78 (1)	10.00		
N-M	0 / 7988	-28.0	-28.0 0.78 (1)	10.00		
M-L	0 / 3491	-28.0	-28.0 0.31 (1)	10.00		
L-K	0 / 0	-28.0	-28.0 0.03 (1)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
Q	10-2-8	-1059	-1059		BACK	VERT	TOTAL
T	10-5-12	-101	-101		BACK	VERT	TOTAL
U	12-6-12	-101	-101		BACK	VERT	TOTAL
V	14-6-12	-101	-101		BACK	VERT	TOTAL
W	16-6-12	-101	-101		BACK	VERT	TOTAL
X	18-3-4	-101	-101		BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.8	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.06")
CALCULATED VERT. DEFL.(LL)= L/989 (0.27")
ALLOWABLE DEFL.(TL)= L/360 (1.06")
CALCULATED VERT. DEFL.(TL)= L/904 (0.42")

CSI: TC=0.31 (D-E:1), BC=0.79 (M-O:1),
WB=0.46 (C-Q:1), SSI=0.34 (H-O:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

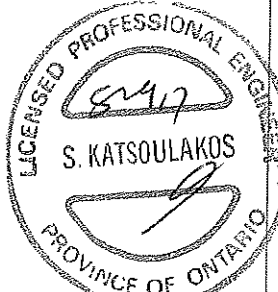
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	618	354	1667
	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (M) (INPUT = 0.90)

JSI METAL= 0.94 (N) (INPUT = 1.00)



DRWG NO. TAM 43635-17
STRUCTURAL
COMPONENT ONLY

JOB NAME 282696	TRUSS NAME T1Z	QUANTITY 1	PLY 2	JOB DESC. TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington					
Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Aug 29 11:22:27 2017 Page 2 ID:WEJRGsZ?LFZ44vkWclgsRFykJvi-38f8YgBZX_3jd5ud3WSZNScHUpRzLQqbrl6tVYyjoVA					

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
S	BMV1+p	MT20	3.0	8.0		

HANGERS NOTES

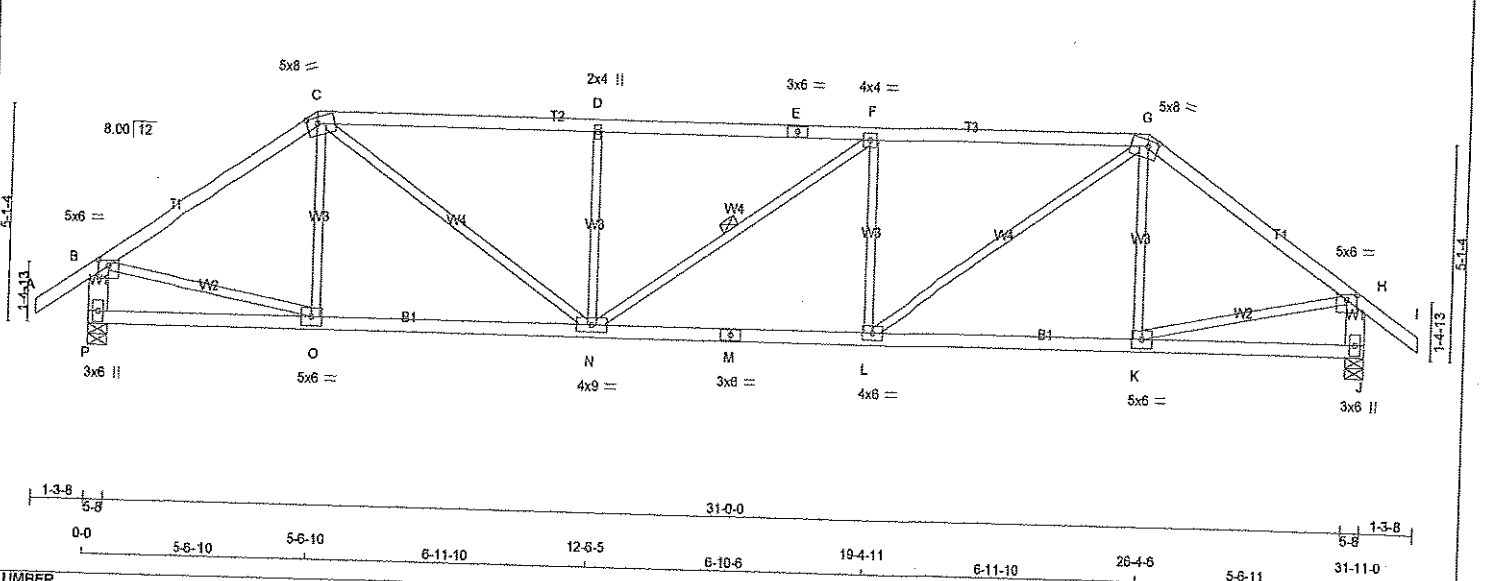
- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 101.4 lbs FACTORED DOWN AT 10-6-12, 101.4 lbs FACTORED DOWN AT 12-6-12, 101.4 lbs FACTORED DOWN AT 14-6-12, AND 101.4 lbs FACTORED DOWN AT 16-6-12, AND 101.4 lbs FACTORED DOWN AT 18-3-4 ON TOP CHORD, AND 1058.6 lbs FACTORED DOWN AT 10-2-8, 69.9 lbs FACTORED DOWN AT 10-8-12, 69.9 lbs FACTORED DOWN AT 12-6-12, 69.9 lbs FACTORED DOWN AT 14-6-12, 69.9 lbs FACTORED DOWN AT 16-6-12, AND 69.9 lbs FACTORED DOWN AT 18-3-4, AND 1580.9 lbs FACTORED DOWN AT 18-7-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
Y	10-6-12	-40	-70	---	BACK	VERT	TOTAL
Z	12-6-12	-40	-70	---	BACK	VERT	TOTAL
AA	14-6-12	-40	-70	---	BACK	VERT	TOTAL
AB	16-6-12	-40	-70	---	BACK	VERT	TOTAL
AC	18-3-4	-40	-70	---	BACK	VERT	TOTAL
AD	18-7-8	-1581	-1581	---	BACK	VERT	TOTAL



DWG NO. TAM 43635.17
 STRUCTURAL
 COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - E	2x4	DRY No.2	SPF
E - G	2x4	DRY No.2	SPF
G - I	2x4	DRY No.2	SPF
P - B	2x6	DRY No.2	SPF
J - H	2x6	DRY No.2	SPF
P - M	2x4	DRY No.2	SPF
M - J	2x4	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-m	MT20	5.0	8.0	1.50	3.00
C	TTWW-m	MT20	5.0	8.0	1.75	3.00
D	TMVW-w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	8.0		
F	TMVW-t	MT20	4.0	4.0		
G	TTWW-m	MT20	5.0	8.0	1.75	3.00
H	TMVW-p	MT20	5.0	6.0	1.50	3.00
J	BMV1+p	MT20	3.0	6.0		
K	BMVW-t	MT20	5.0	8.0		
L	BMVW-t	MT20	4.0	6.0		
M	BS-t	MT20	3.0	8.0		
N	BMVW-t	MT20	4.0	9.0		
O	BMVW-t	MT20	5.0	6.0		
P	BMV1+p	MT20	3.0	8.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	FACTORED GROSS REACTION HORZ	INPUT BRG IN-SX	REQD BRG IN-SX
P	1907	0	5-8	5-8
J	1907	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	MAX. MIN. SNOW	MAX. MIN. LIVE	PERM. LIVE	WIND	DEAD	SOIL
P	1549	887 / 0	335 / 0	0 / 0	0 / 0	327 / 0	0 / 0
J	1549	887 / 0	335 / 0	0 / 0	0 / 0	327 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.22 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (L)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (L)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	0-C	-136 / 189
B-C	-2095 / 0	-84.3	-84.3	0.63 (1)	4.04	C-N	0 / 1450
C-D	-2932 / 0	-84.3	-84.3	0.92 (1)	3.22	N-D	-632 / 0
D-E	-2932 / 0	-84.3	-84.3	0.91 (1)	3.22	N-F	-1 / 0
E-F	-2932 / 0	-84.3	-84.3	0.91 (1)	3.22	L-F	-632 / 0
F-G	-2933 / 0	-84.3	-84.3	0.93 (1)	3.22	L-G	0 / 1452
G-H	-2094 / 0	-84.3	-84.3	0.63 (1)	4.04	K-G	-136 / 189
H-I	0 / 32	-84.3	-84.3	0.11 (1)	10.00	B-O	0 / 1772
P-B	-1847 / 0	0.0	0.0	0.12 (1)	7.35	K-H	0 / 1772
J-H	-1847 / 0	0.0	0.0	0.12 (1)	7.35		
P-O	0 / 0	-28.0	-28.0	0.26 (3)	10.00		
O-N	0 / 1738	-28.0	-28.0	0.47 (2)	10.00		
N-M	0 / 2933	-28.0	-28.0	0.60 (1)	10.00		
M-L	0 / 2933	-28.0	-28.0	0.60 (1)	10.00		
L-K	0 / 1738	-28.0	-28.0	0.47 (2)	10.00		
K-J	0 / 0	-28.0	-28.0	0.26 (3)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 3.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014

- CSA 086-09

- TPIG 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.08")

CALCULATED VERT. DEFL.(LL) = L/999 (0.20")

ALLOWABLE DEFL.(TL) = L/360 (1.08")

CALCULATED VERT. DEFL.(TL) = L/999 (0.33")

CSI: TC=0.93 (F-G:1), BC=0.60 (L-N:1), WB=0.40 (B-O:1), SSI=0.27 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

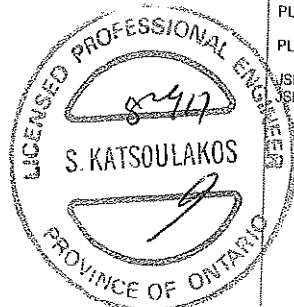
MT20 618 354 1657 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

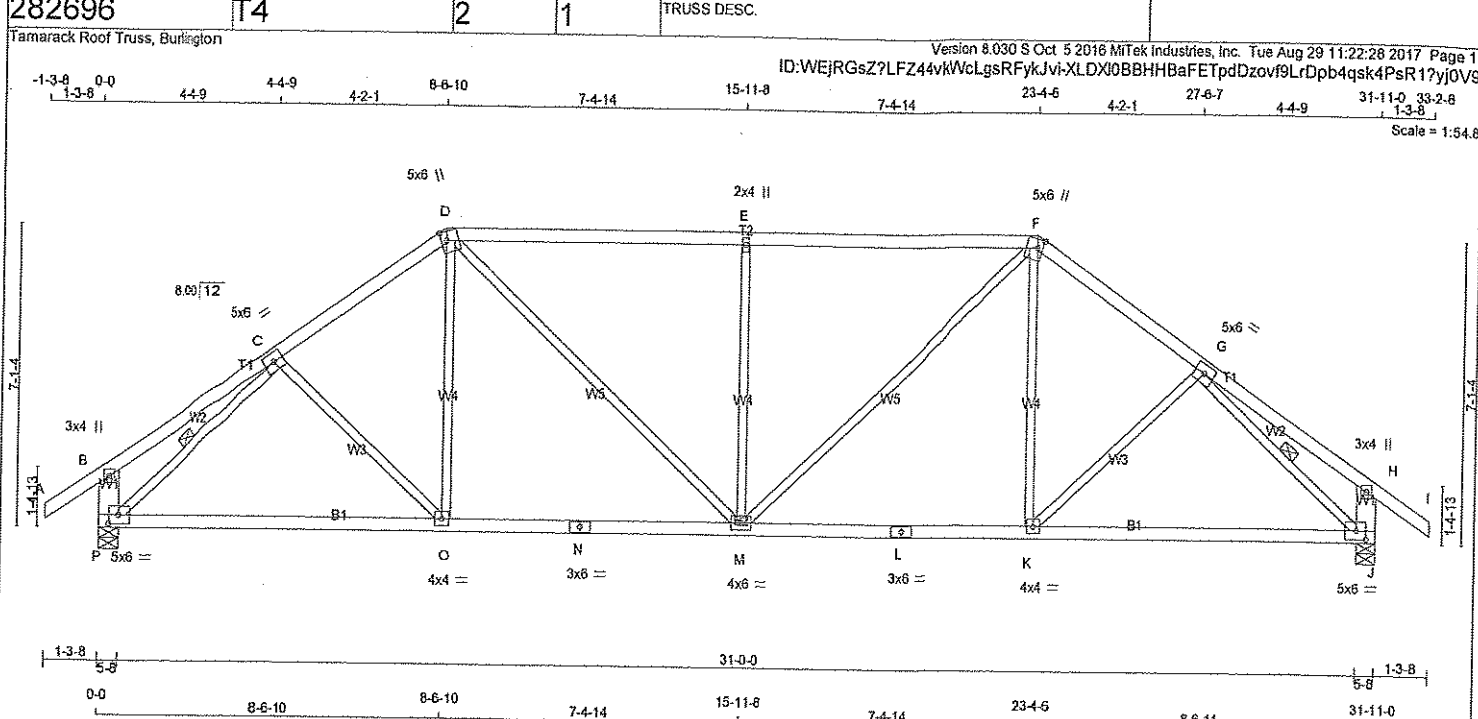
JSI GRIP= 0.88 (M) (INPUT = 0.60)

JSI METAL= 0.89 (M) (INPUT = 1.00)



DWG NO. TAM43636-17

STRUCTURAL COMPONENT ONLY



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
P - B	2x6	DRY	No.2
J - H	2x6	DRY	No.2
P - N	2x4	DRY	No.2
N - L	2x4	DRY	No.2
L - J	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	5.0	6.0		
D	TTVWV+m	MT20	5.0	6.0	2.50	1.50
E	TMVW+w	MT20	2.0	4.0		
F	TTVWV+m	MT20	5.0	6.0	2.50	1.50
G	TMVW-t	MT20	5.0	6.0		
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	5.0	6.0	2.50	2.75
K	BMVW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMVWV-t	MT20	4.0	6.0		
N	BS-t	MT20	3.0	6.0		
O	BMVW-t	MT20	4.0	4.0		
P	BMVW1-t	MT20	5.0	6.0	2.50	2.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
P	1907	0	1907	0	5-8	5-8
J	1907	0	1907	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM LIVE			
P	1549	887 / 0	335 / 0	0 / 0	0 / 0	327 / 0	0 / 0
J	1549	887 / 0	335 / 0	0 / 0	0 / 0	327 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLINE SPACING = 3.74 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-P, G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 32	-84.3 -84.3	0.11 (1)	10.00	C-O	-15 / 137 0.03 (3)	
B-C	0 / 22	-84.3 -84.3	0.23 (1)	10.00	O-D	0 / 391 0.09 (2)	
C-D	-2041 / 0	-84.3 -84.3	0.24 (1)	4.55	D-M	0 / 692 0.16 (1)	
D-E	-2191 / 0	-84.3 -84.3	0.72 (1)	3.74	M-E	-767 / 0 0.67 (1)	
E-F	-2191 / 0	-84.3 -84.3	0.72 (1)	3.74	M-F	0 / 692 0.16 (1)	
F-G	-2041 / 0	-84.3 -84.3	0.24 (1)	4.55	K-F	0 / 391 0.08 (2)	
G-H	0 / 22	-84.3 -84.3	0.23 (1)	10.00	K-G	-15 / 137 0.03 (3)	
H-I	0 / 32	-84.3 -84.3	0.11 (1)	10.00	P-C	-2294 / 0 0.54 (1)	
P-B	-256 / 0	0.0	0.0 0.02 (1)	7.81	G-J	-2294 / 0 0.54 (1)	
J-H	-256 / 0	0.0	0.0 0.02 (1)	7.81			
P-O	0 / 1688	-28.0 -28.0	0.68 (2)	10.00			
O-N	0 / 1682	-28.0 -28.0	0.69 (2)	10.00			
N-M	0 / 1682	-28.0 -28.0	0.69 (2)	10.00			
M-L	0 / 1682	-28.0 -28.0	0.69 (2)	10.00			
L-K	0 / 1682	-28.0 -28.0	0.69 (2)	10.00			
K-J	0 / 1688	-28.0 -28.0	0.68 (2)	10.00			

TOTAL WEIGHT = 2 X 134 = 268 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.06")
CALCULATED VERT. DEFL.(LL) = L/999 (0.23")
ALLOWABLE DEFL.(TL) = L/360 (1.06")
CALCULATED VERT. DEFL.(TL) = L/999 (0.38")

CSI: TC=0.72 (E-F:1), BC=0.69 (K-M:2), WB=0.67 (E-M:1), SS=0.30 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

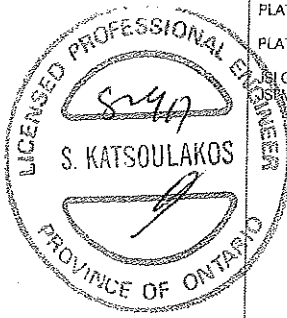
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 822 2284 1656

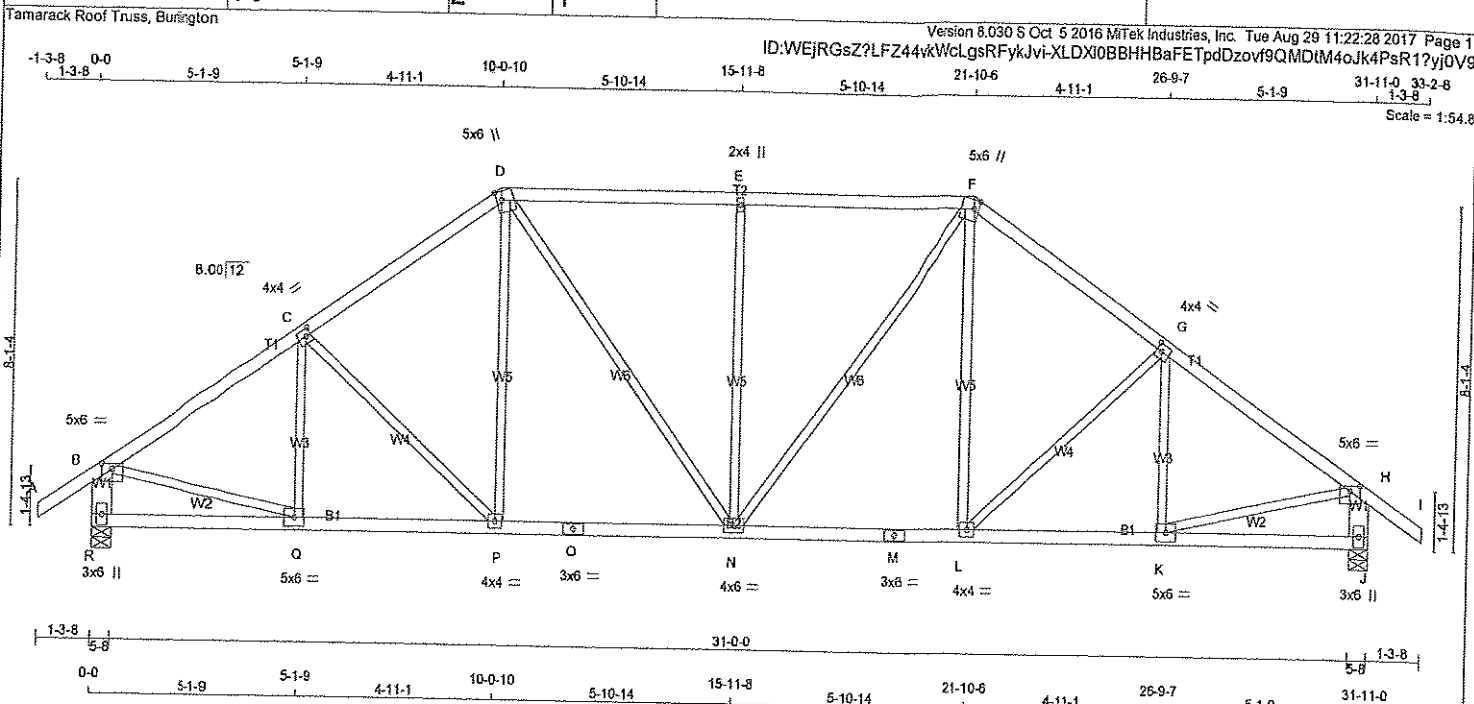
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

GRIP= 0.89 (D) (INPUT = 0.90)
SS METAL= 0.57 (C) (INPUT = 1.00)



DWG NO. TAM 4363817
STRUCTURAL
COMPONENT ONLY



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
R - B	2x6	DRY	No.2
J - H	2x6	DRY	No.2
R - O	2x4	DRY	No.2
O - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVV-p	MT20	5.0	6.0	1.50	3.00
C	TMVV-t	MT20	4.0	4.0	2.00	1.50
D	TTVVW+m	MT20	5.0	6.0	2.50	1.50
E	TMVW-w	MT20	2.0	4.0		
F	TTVVW+m	MT20	5.0	6.0	2.50	1.50
G	TMVVW-t	MT20	4.0	4.0	2.00	1.50
H	TMVVW-p	MT20	5.0	6.0	1.50	3.00
J	BMV1+p	MT20	3.0	6.0		
K	BMVVW-t	MT20	5.0	6.0		
L	BMVVW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMVVW-t	MT20	4.0	6.0		
O	BS-t	MT20	3.0	6.0		
P	BMVVW-t	MT20	4.0	4.0		
Q	BMVVW-t	MT20	5.0	6.0		
R	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	IN-SX	IN-SX
R	1907	0	5-8	5-8
J	1907	0	5-8	5-8

UNFACTORED REACTIONS

	1ST CASE	MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
R	1549	887 / 0	335 / 0	0 / 0	0 / 0	327 / 0	0 / 0
J	1549	887 / 0	335 / 0	0 / 0	0 / 0	327 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.37 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	
FR-TO				FR-TO			
A-B	0 / 32	-84.3	-84.3 0.11 (1)	10.00	Q-C	-224 / 86	0.08 (1)
B-C	-2114 / 0	-84.3	-84.3 0.34 (1)	4.37	C-P	-251 / 0	0.22 (1)
C-D	-1939 / 0	-84.3	-84.3 0.33 (1)	4.54	P-D	0 / 377	0.09 (2)
D-E	-1884 / 0	-84.3	-84.3 0.43 (1)	4.44	D-N	0 / 485	0.11 (1)
E-F	-1884 / 0	-84.3	-84.3 0.43 (1)	4.44	N-E	-608 / 0	0.77 (1)
F-G	-1939 / 0	-84.3	-84.3 0.33 (1)	4.54	N-F	0 / 485	0.11 (1)
G-H	-2114 / 0	-84.3	-84.3 0.34 (1)	4.37	F-G	0 / 377	0.08 (2)
H-I	0 / 32	-84.3	-84.3 0.11 (1)	10.00	L-G	-261 / 0	0.22 (1)
R-B	-1846 / 0	0.0	0.0 0.12 (1)	7.35	K-G	-224 / 86	0.08 (1)
J-H	-1846 / 0	0.0	0.0 0.12 (1)	7.35	B-Q	0 / 1821	0.41 (1)
					K-H	0 / 1821	0.41 (1)
R-Q	0 / 0	-28.0	-28.0 0.16 (3)	10.00			
Q-P	0 / 1782	-28.0	-28.0 0.39 (1)	10.00			
P-O	0 / 1592	-28.0	-28.0 0.39 (2)	10.00			
O-N	0 / 1592	-28.0	-28.0 0.39 (2)	10.00			
N-M	0 / 1592	-28.0	-28.0 0.39 (2)	10.00			
M-L	0 / 1592	-28.0	-28.0 0.39 (2)	10.00			
L-K	0 / 1782	-28.0	-28.0 0.39 (1)	10.00			
K-J	0 / 0	-28.0	-28.0 0.16 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 0.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.08")
CALCULATED VERT. DEFL. (LL) = L/999 (0.10")
ALLOWABLE DEFL. (TL) = L/360 (1.08")
CALCULATED VERT. DEFL. (TL) = L/999 (0.17")

CSI: TC=0.43 (D-E:1), BC=0.39 (L-N:2), WB=0.77 (E-N:1), SS=0.24 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 822 2284 1656

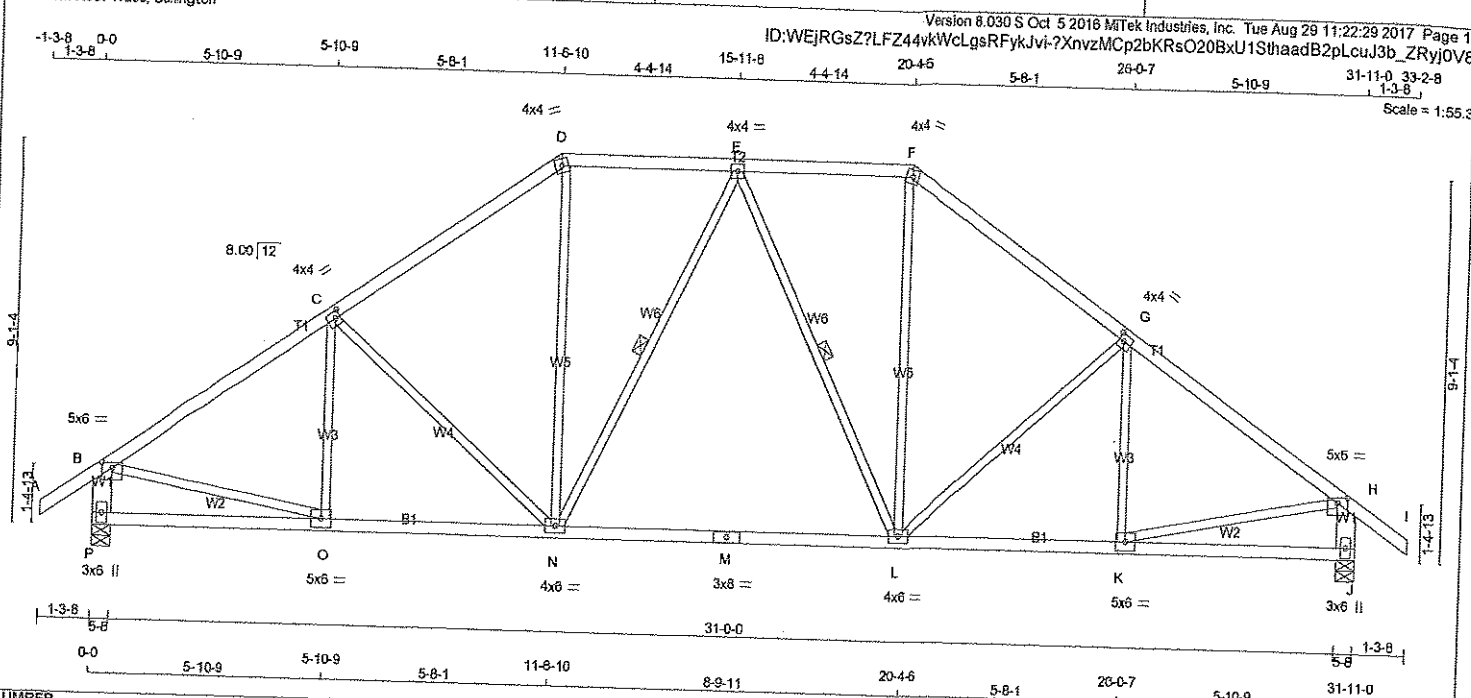
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSRIP = 0.90 (Q) (INPUT = 0.90)
JSRMETAL = 0.47 (O) (INPUT = 1.00)



DWG NO. TAM 43639-17
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
P - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG UPLIFT IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
P	1907	0	1907	0	5-8	5-8
J	1907	0	1907	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS				
		SNOW	LIVE	PERM. LIVE	WIND	DEAD
P	1549	887 / 0	335 / 0	0 / 0	0 / 0	327 / 0
J	1549	887 / 0	335 / 0	0 / 0	0 / 0	327 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.22 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N, E-L

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)
FR-TO		FROM TO	FR-TO	FROM TO
A-B	0 / 32	-84.3 -84.3 0.11 (1)	O-C	-169 / 125 0.07 (1)
B-C	-2130 / 0	-84.3 -84.3 0.46 (1)	C-N	-378 / 0 0.45 (1)
C-D	-1853 / 0	-84.3 -84.3 0.42 (1)	N-D	0 / 694 0.15 (1)
D-E	-1521 / 0	-84.3 -84.3 0.22 (1)	E-L	-248 / 0 0.17 (1)
E-F	-1521 / 0	-84.3 -84.3 0.22 (1)	L-F	0 / 694 0.15 (1)
F-G	-1853 / 0	-84.3 -84.3 0.42 (1)	G-H	-378 / 0 0.45 (1)
G-H	-2130 / 0	-84.3 -84.3 0.46 (1)	H-I	-169 / 125 0.07 (1)
H-I	0 / 32	-84.3 -84.3 0.11 (1)	I-O	0 / 1829 0.41 (1)
P-B	-1836 / 0	0.0 0.0 0.12 (1)	B-O	0 / 1829 0.41 (1)
J-H	-1836 / 0	0.0 0.0 0.12 (1)	K-H	0 / 1829 0.41 (1)
P-O	0 / 0	-28.0 -28.0 0.20 (2)		
O-N	0 / 1799	-28.0 -28.0 0.55 (2)		
N-M	0 / 1630	-28.0 -28.0 0.53 (2)		
M-L	0 / 1630	-28.0 -28.0 0.53 (2)		
L-K	0 / 1799	-28.0 -28.0 0.55 (2)		
K-J	0 / 0	-28.0 -28.0 0.20 (2)		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.06")
CALCULATED VERT. DEFL.(LL) = L/999 (0.19")
ALLOWABLE DEFL.(TL) = L/360 (1.06")
CALCULATED VERT. DEFL.(TL) = L/999 (0.31")

CSI: TC=0.46 (G-H:1), BC=0.55 (K-L:2), WB=0.45 (G-L:1), SSI=0.20 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

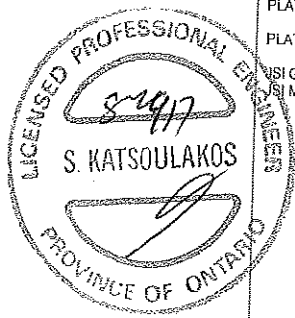
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1657 822 2284 1656

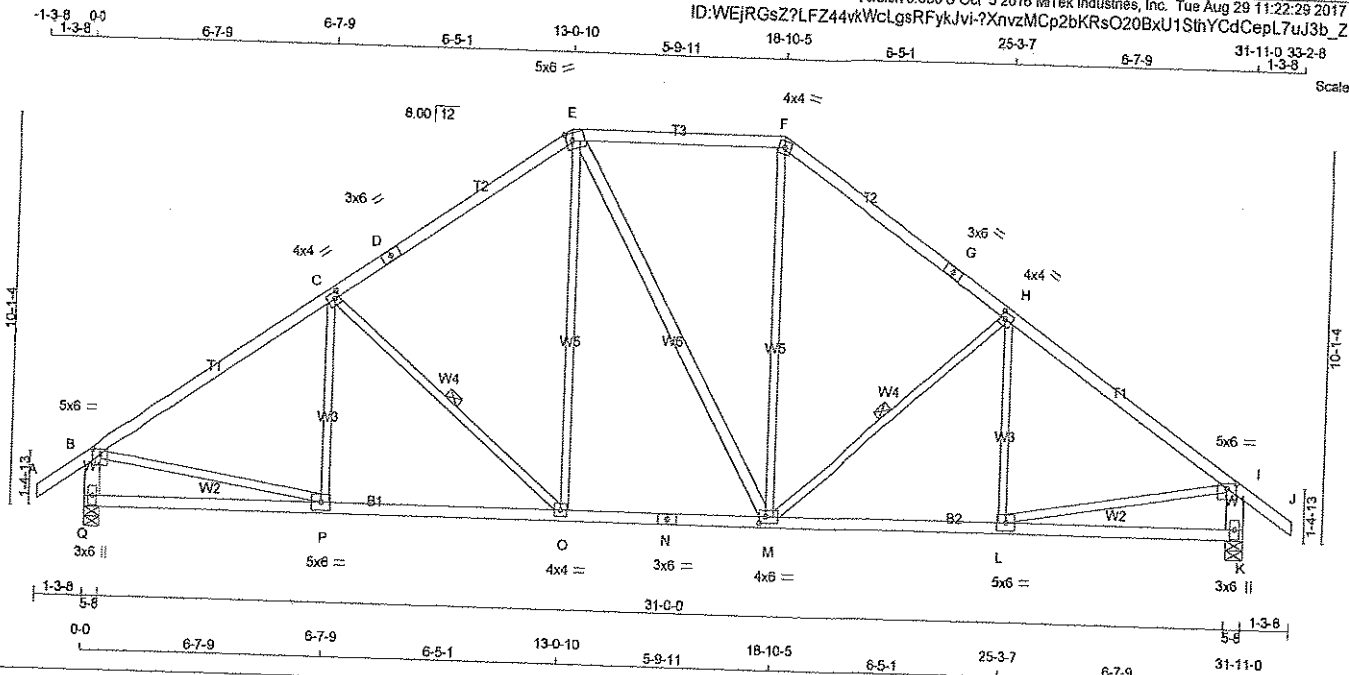
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SSI GRIP= 0.87 (O) (INPUT = 0.90)
SSI METAL= 0.77 (M) (INPUT = 1.00)



DRWG NO. TAM 4364217
STRUCTURAL
COMPONENT ONLY



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
G - J	2x4	DRY	No.2
Q - B	2x6	DRY	No.2
K - I	2x6	DRY	No.2
Q - N	2x4	DRY	No.2
N - K	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
E - M	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVV-p	MT20	5.0	6.0	1.50	3.00
C	TMVV-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTVV-m	MT20	5.0	6.0	2.25	2.00
F	TTVV-m	MT20	4.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMVV-t	MT20	4.0	4.0	2.00	1.50
I	TMVV-p	MT20	5.0	6.0	1.50	3.00
K	BMV1+p	MT20	3.0	6.0		
L	BMVV-t	MT20	5.0	6.0		
M	BMVVW-t	MT20	4.0	6.0	2.00	2.00
N	BS-t	MT20	3.0	6.0		
O	BMVV-t	MT20	4.0	4.0		
P	BMVV-t	MT20	5.0	6.0		
Q	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
Q	1907	0	5-8	5-8
K	1907	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	COMBINED	1549	887 / 0	335 / 0	0 / 0	0 / 0	327 / 0	0 / 0
K	COMBINED	1549	887 / 0	335 / 0	0 / 0	0 / 0	327 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.03 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-O, H-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 32	-84.3 -84.3	0.11 (1)	10.00	P-C	-87 / 222	0.05 (3)
B-C	-2140 / 0	-84.3 -84.3	0.62 (1)	4.03	C-O	-520 / 0	0.26 (1)
C-D	-1738 / 0	-84.3 -84.3	0.55 (1)	4.45	O-E	0 / 519	0.12 (2)
D-E	-1738 / 0	-84.3 -84.3	0.55 (1)	4.45	E-M	0 / 1	0.00 (2)
E-F	-1420 / 0	-84.3 -84.3	0.40 (1)	5.01	M-F	0 / 520	0.12 (2)
F-G	-1739 / 0	-84.3 -84.3	0.55 (1)	4.45	M-H	-519 / 0	0.26 (1)
G-H	-1739 / 0	-84.3 -84.3	0.55 (1)	4.45	L-H	-88 / 221	0.05 (3)
H-I	-2140 / 0	-84.3 -84.3	0.61 (1)	4.03	B-P	0 / 1835	0.41 (1)
I-J	0 / 32	-84.3 -84.3	0.11 (1)	10.00	L-I	0 / 1835	0.41 (1)
Q-B	-1832 / 0	0.0	0.0	0.12 (1)			
K-I	-1831 / 0	0.0	0.0	0.12 (1)			
Q-P	0 / 0	-28.0	-28.0	0.30 (3)			
P-O	0 / 1811	-28.0	-28.0	0.51 (2)			
O-N	0 / 1419	-28.0	-28.0	0.38 (2)			
N-M	0 / 1419	-28.0	-28.0	0.35 (2)			
M-L	0 / 1811	-28.0	-28.0	0.51 (2)			
L-K	0 / 0	-28.0	-28.0	0.30 (3)			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 45.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.06")
CALCULATED VERT. DEFL.(LL) = L/999 (0.10")
ALLOWABLE DEFL.(TL) = L/360 (1.06")
CALCULATED VERT. DEFL.(TL) = L/999 (0.17")

CSI: TC=0.82 (B-C:1), BC=0.51 (O-P:2), WB=0.41 (B-P:1), SSI=0.22 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

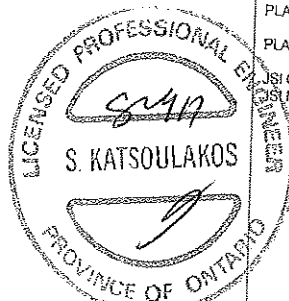
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1655

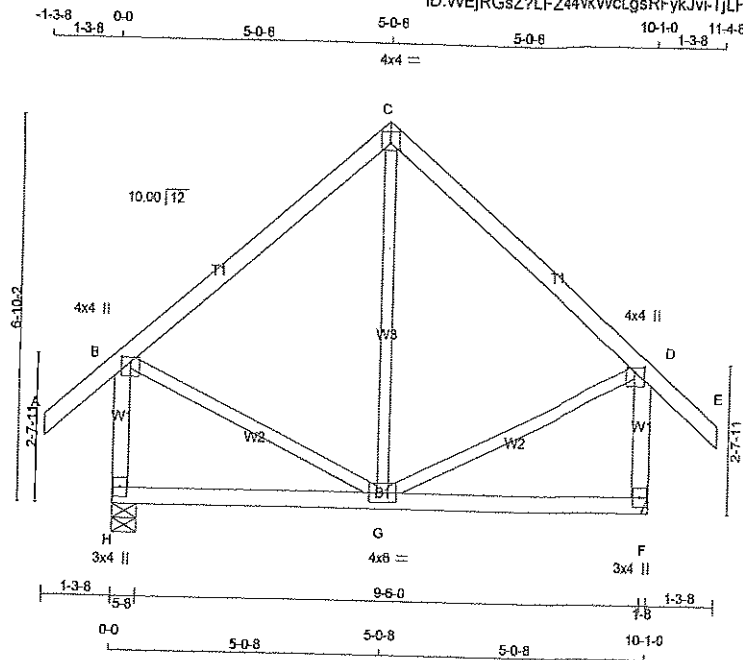
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.86 (P) (INPUT = 0.90)
JSI METAL= 0.45 (P) (INPUT = 1.00)



DRWG NO. TAM 43641-17
STRUCTURAL
COMPONENT ONLY



Scale = 1:41.2

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
H - B	2x4	DRY	No.2
F - D	2x4	DRY	No.2
H - F	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW-p	MT20	4.0	4.0	1.50	2.00
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMVWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
H	683	0	683	0	5-8	5-8
F	683	0	683	0	5-8	5-8

HANGER BY OTHERS
MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS

JT	COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	544	329 / 0	106 / 0	0 / 0	0 / 0	109 / 0	0 / 0
F	544	329 / 0	106 / 0	0 / 0	0 / 0	109 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

FR-TO	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. UNBRACED LENGTH (FT)	MAX. FACTORED FORCE (LBS)
A-B	0 / 37	-84.3	10.00	G-C
B-C	-302 / 0	-84.3	6.25	B-G
C-D	-302 / 0	-84.3	6.25	G-D
D-E	0 / 37	-84.3	10.00	
H-B	-529 / 0	0.0	7.81	
F-D	-629 / 0	0.0	7.81	
H-G	0 / 0	-28.0	10.00	
G-F	0 / 0	-28.0	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.34")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.02")
ALLOWABLE DEFL.(TL) = $L/360$ (0.34")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.03")

CSI: TC=0.28 (B-C:1), BC=0.21 (G-H:3), WB=0.06 (D-G:1), SSI=0.13 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

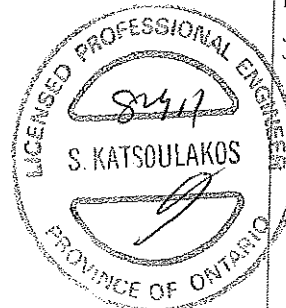
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1667 822 2284 1656

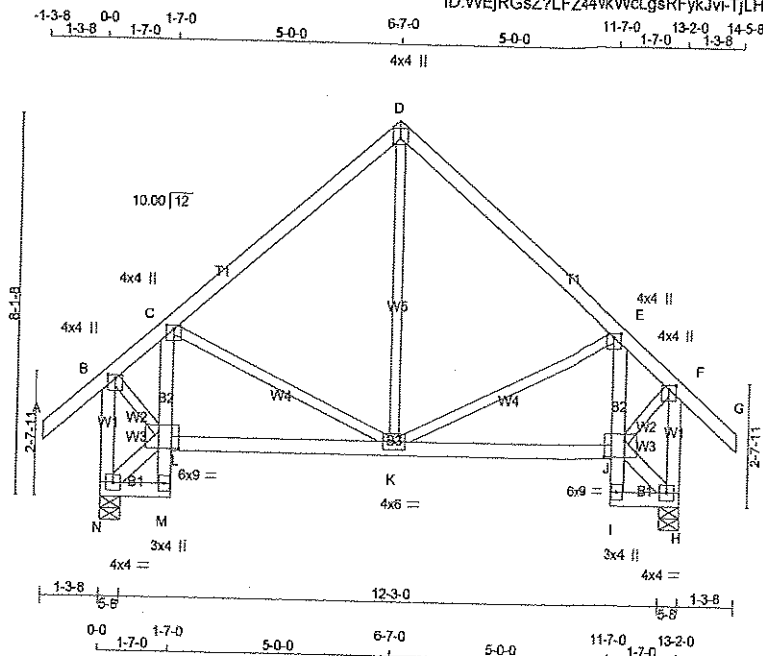
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.62 (C) (INPUT = 0.90)
JSI METAL= 0.14 (D) (INPUT = 1.00)



DRWG NO. TAM 43643-17
STRUCTURAL
COMPONENT ONLY



Scale = 1:49.7

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
N - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
N - M	2x4	DRY	No.2
M - C	2x4	DRY	No.2
L - J	2x4	DRY	No.2
I - E	2x4	DRY	No.2
I - H	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
N - L	2x4	DRY	No.2
J - H	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B, C, E, F				
B	TMVW+p	MT20	4.0	4.0 1.00 2.00
D	TIW+p	MT20	4.0	4.0 1.50 2.00
H	BMVW1-t	MT20	4.0	4.0
I	BMV+p	MT20	3.0	4.0
J	BMVWW-I	MT20	6.0	9.0 4.25 5.50
K	BMVWW-t	MT20	4.0	8.0
L	BMVWW-I	MT20	6.0	9.0 4.25 5.50
M	BMV+p	MT20	3.0	4.0
N	BMVW1-t	MT20	4.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	UPLIFT	IN-SX
N	856	0	856	0	5-8
H	856	0	856	0	5-8

UNFACTORED REACTIONS		1ST CASE	MAX/MIN COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
N	686	408 / 0	138 / 0
H	686	408 / 0	138 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FR-TO)	MAX. FACTORED FORCE (LBS)
FR-TO				FR-TO			
A-B	0 / 37	-84.3	-84.3 0.12 (1)	10.00	K-D	0 / 328	0.07 (2)
B-C	-537 / 0	-84.3	-84.3 0.22 (1)	6.25	K-E	-114 / 0	0.09 (1)
C-D	-493 / 0	-84.3	-84.3 0.26 (1)	6.25	C-K	-114 / 0	0.06 (1)
D-E	-493 / 0	-84.3	-84.3 0.26 (1)	6.25	N-L	-16 / 0	0.09 (1)
E-F	-537 / 0	-84.3	-84.3 0.22 (1)	6.25	B-L	0 / 578	0.13 (1)
F-G	0 / 37	-84.3	-84.3 0.12 (1)	10.00	J-H	-16 / 0	0.09 (1)
N-B	-825 / 0	0.0	0.0 0.12 (1)	7.81	J-F	0 / 578	0.13 (1)
H-F	-825 / 0	0.0	0.0 0.12 (1)	7.81			
N-M	0 / 12	-28.0	-28.0 0.02 (2)	10.00			
M-L	0 / 35	0.0	0.0 0.02 (1)	10.00			
L-C	-274 / 0	0.0	0.0 0.02 (1)	7.81			
L-K	0 / 461	-28.0	-28.0 0.27 (2)	10.00			
K-J	0 / 461	-28.0	-28.0 0.27 (2)	10.00			
I-J	0 / 35	0.0	0.0 0.02 (1)	10.00			
J-E	-274 / 0	0.0	0.0 0.02 (1)	7.81			
I-H	0 / 12	-28.0	-28.0 0.02 (2)	10.00			

TOTAL WEIGHT = 3 X 71 = 214 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.44")
CALCULATED VERT. DEFL. (LL) = $L/899$ (0.03")
ALLOWABLE DEFL. (TL) = $L/360$ (0.44")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.04")
CSI: TC=0.26 (C-D:1), BC=0.27 (K-L:2), WB=0.13 (F-J:1), SSI=0.15 (C-D:1)

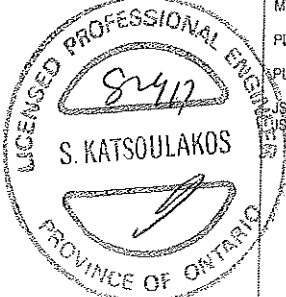
DOL LUMBER=1.60 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10
COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

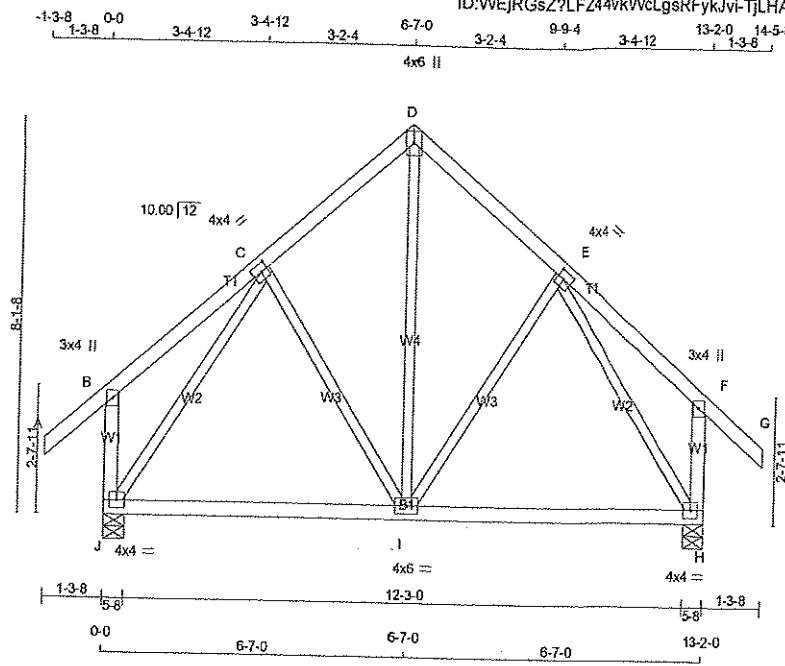
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.

CSI GRIP= 0.87 (D) (INPUT = 0.90)
CSI METAL= 0.17 (B) (INPUT = 1.00)



DRWG NO. TAM 43644-17
STRUCTURAL
COMPONENT ONLY



Scale: 1/4"=1'

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
J - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
J - H	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2

EXCEPT

SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
J	856	0	856	0	5-8	5-8
H	856	0	856	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE		MAX/MIN COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
J	886	408 / 0	138 / 0	0 / 0	0 / 0	140 / 0	0 / 0
H	886	408 / 0	138 / 0	0 / 0	0 / 0	140 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		MAX. FACTORED	MEMB.	WEBS		FACTORED		MAX. FACTORED
	FORCE (LBS)	VERT. LOAD (LBS)	LC1 (PLF)	CS1 (LC)			FORCE (LBS)	CS1 (LC)	FORCE (LBS)	CS1 (LC)	
FR-TO						FR-TO					
A-B	0 / 37	-84.3	-84.3	0.12 (1)	10.00	I-D	0 / 349	-63 / 64	0.08 (2)	0.04 (1)	
B-C	0 / 20	-84.3	-84.3	0.14 (1)	10.00	I-E	-63 / 64	-63 / 64	0.04 (1)	0.04 (1)	
C-D	-451 / 0	-84.3	-84.3	0.11 (1)	6.25	C-I	-63 / 64	-63 / 64	0.04 (1)	0.04 (1)	
D-E	-451 / 0	-84.3	-84.3	0.11 (1)	6.25	J-C	-672 / 0	-672 / 0	0.41 (1)	0.41 (1)	
E-F	0 / 20	-84.3	-84.3	0.14 (1)	10.00	E-H	-672 / 0	-672 / 0	0.41 (1)	0.41 (1)	
F-G	0 / 37	-84.3	-84.3	0.12 (1)	10.00						
J-B	-222 / 0	0.0	0.0	0.03 (1)	7.81						
H-F	-222 / 0	0.0	0.0	0.03 (1)	7.81						
J-I	0 / 368	-28.0	-28.0	0.39 (2)	10.00						
I-H	0 / 368	-28.0	-28.0	0.39 (2)	10.00						

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	4.0	2.00	1.75
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWW1-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

TOTAL WEIGHT = 4 X 69 = 274 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 3.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, CBC 2012, ABC 2014

- CSA 086-09

- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.44")

CALCULATED VERT. DEFL. (LL) = L/999 (0.06")

ALLOWABLE DEFL. (TL) = L/360 (0.44")

CALCULATED VERT. DEFL. (TL) = L/999 (0.09")

CSI: TC=0.14 (B-C:1), BC=0.39 (H-I:2), WB=0.41 (C-J:1), SSI=0.16 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

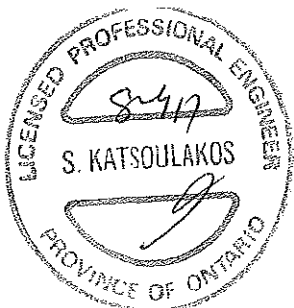
PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1687 822 2284 1856

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

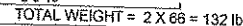
JSI GRIP= 0.81 (E) (INPUT = 0.90)

JSI METAL= 0.24 (E) (INPUT = 1.00)



DWG NO. TAM 43645-17

STRUCTURAL COMPONENT ONLY



LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE
2-9-10	-1	-2	105	FRONT	VERT	TOTAL
11-0-6	-1	-2	105	FRONT	VERT	TOTAL
10-11-0	8	—	8	FRONT	VERT	TOTAL
7-11-4	-1879	-1879	—	BACK	VERT	TOTAL
8-0-0	-2846	-2846	—	BACK	VERT	TOTAL
6-4-8	8	—	8	FRONT	VERT	TOTAL
2-10-6	8	—	8	FRONT	VERT	TOTAL
4-10-6	-1	-2	106	FRONT	VERT	TOTAL
6-4-8	-1	-2	106	FRONT	VERT	TOTAL
7-5-10	-1	-2	106	FRONT	VERT	TOTAL
8-11-10	-1	-2	106	FRONT	VERT	TOTAL
10-6	8	—	8	FRONT	VERT	TOTAL
4-10-6	8	—	8	FRONT	VERT	TOTAL

DWG NO. TAM 43646 11
STRUCTURAL
COMPONENT ONLY



JOB NAME 282696	TRUSS NAME T12	QUANTITY 1	PLY 2	JOB DESC. TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington					
Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Aug 29 11:22:31 2017 Page 2 ID:WEJRGsZ?LFZ44vKwLgSRFykJvi-xwvFO2E3aCa96iCOILVVXInvFQnpHBtAmN45eJyjQV6					

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1.9 lbs FACTORED DOWN AND 105.4 lbs FACTORED UP AT 2-9-10, 1.7 lbs FACTORED DOWN AND 105.7 lbs FACTORED UP AT 4-10-6, 1.7 lbs FACTORED DOWN AND 105.7 lbs FACTORED UP AT 6-4-6, 1.7 lbs FACTORED DOWN AND 105.7 lbs FACTORED UP AT 7-5-10, AND 1.7 lbs FACTORED DOWN AND 105.7 lbs FACTORED UP AT 8-11-10, AND 1.9 lbs FACTORED DOWN AND 105.4 lbs FACTORED UP AT 11-0-8 ON TOP CHORD, AND 7.5 lbs FACTORED UP AT 10-6, 7.5 lbs FACTORED UP AT 2-10-6, 7.5 lbs FACTORED UP AT 4-10-6, 2845.9 lbs FACTORED DOWN AT 6-0-0, 7.5 lbs FACTORED UP AT 6-4-6, 7.5 lbs FACTORED UP AT 7-5-10, 1879.0 lbs FACTORED DOWN AT 7-11-4, 7.5 lbs FACTORED UP AT 8-11-10, 1879.0 lbs FACTORED DOWN AT 9-11-4, 7.5 lbs FACTORED UP AT 10-11-10, AND 1879.0 lbs FACTORED DOWN AT 11-11-4, AND 7.5 lbs FACTORED UP AT 12-11-10 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
S	7-5-10	8	—	8	FRONT	VERT	TOTAL
T	8-11-10	8	—	8	FRONT	VERT	TOTAL
U	9-11-4	-1879	-1879	—	BACK	VERT	TOTAL
V	11-11-4	-1879	-1879	—	BACK	VERT	TOTAL
W	12-11-10	8	—	8	FRONT	VERT	TOTAL



DWG NO. TAM 4364617
STRUCTURAL
COMPONENT ONLY

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
T	BMWW-t	MT20	5.0	6.0	2.50	2.75
U	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 472.3 lbs FACTORED DOWN AT 5-10-8, 101.4 lbs FACTORED DOWN AT 7-11-4, 101.4 lbs FACTORED DOWN AT 9-11-4, 101.4 lbs FACTORED DOWN AT 11-11-4, 101.4 lbs FACTORED DOWN AT 13-11-4, 101.4 lbs FACTORED DOWN AT 15-11-8, 101.4 lbs FACTORED DOWN AT 17-11-12, 101.4 lbs FACTORED DOWN AT 19-11-12, 101.4 lbs FACTORED DOWN AT 21-11-12, AND 101.4 lbs FACTORED DOWN AT 23-11-12, AND 104.9 lbs FACTORED DOWN AT 26-0-8 ON TOP CHORD, AND 84.6 lbs FACTORED DOWN AT 1-11-4, 84.6 lbs FACTORED DOWN AT 3-11-4, 69.9 lbs FACTORED DOWN AT 5-11-4, 69.9 lbs FACTORED DOWN AT 7-11-4, 69.9 lbs FACTORED DOWN AT 9-11-4, 69.9 lbs FACTORED DOWN AT 11-11-4, 69.9 lbs FACTORED DOWN AT 13-11-4, 69.9 lbs FACTORED DOWN AT 15-11-8, 69.9 lbs FACTORED DOWN AT 17-11-12, 69.9 lbs FACTORED DOWN AT 19-11-12, 69.9 lbs FACTORED DOWN AT 21-11-12, 69.9 lbs FACTORED DOWN AT 23-11-12, 69.9 lbs FACTORED DOWN AT 25-11-12, AND 69.9 lbs FACTORED DOWN AT 27-11-12, AND 69.9 lbs FACTORED DOWN AT 29-11-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LOADING

TOTAL LOAD CASES: (4)

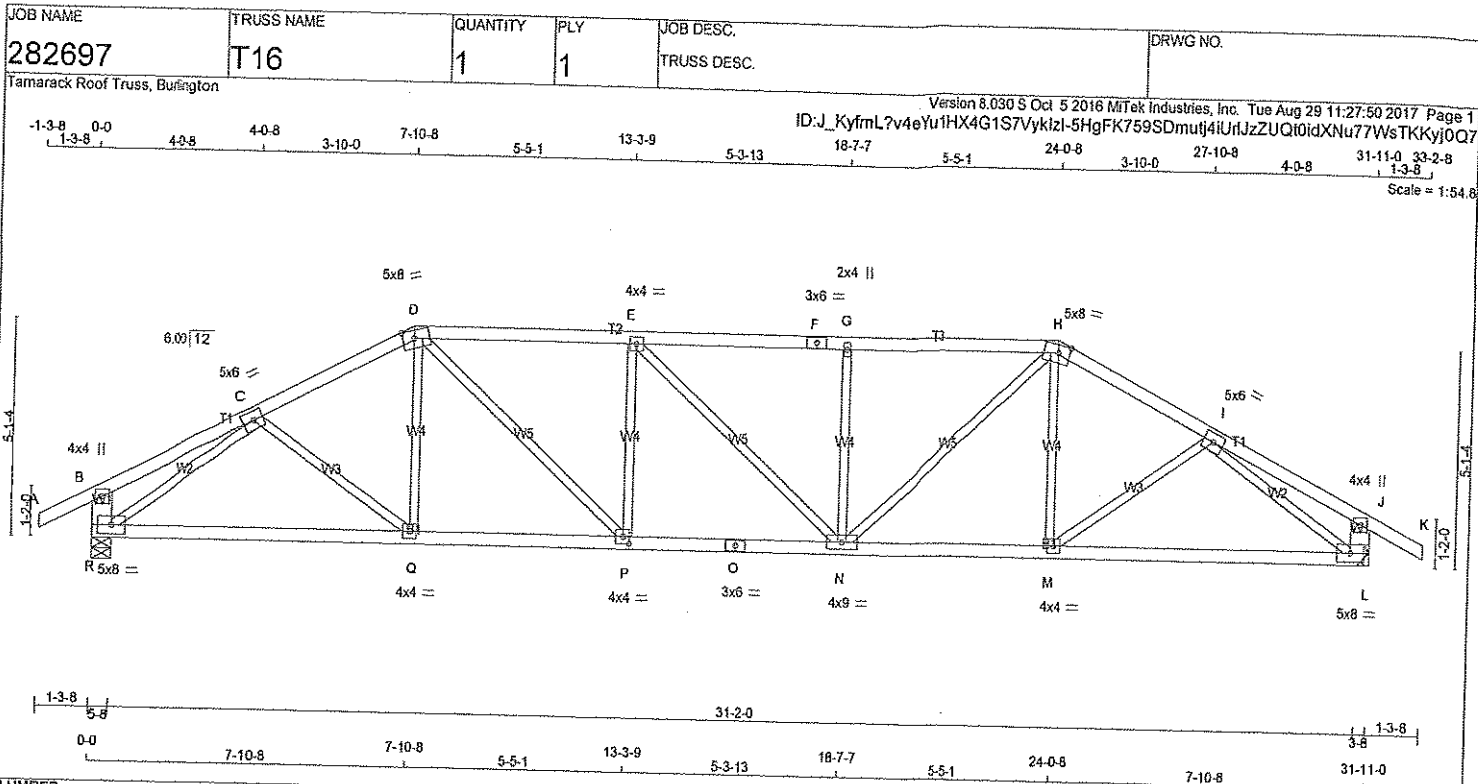
CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
AM-L	0/0	-28.0 -28.0	0.05 (2)	10.69			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	D/R	TYPE
C	5-10-8	-472	-472		FRONT	VERT	TOTAL
E	15-11-8	-101	-101		FRONT	VERT	TOTAL
H	26-0-8	-105	-105		FRONT	VERT	TOTAL
N	25-11-12	-40	-70		FRONT	VERT	TOTAL
P	19-11-12	-40	-70		FRONT	VERT	TOTAL
Q	15-11-8	-40	-70		FRONT	VERT	TOTAL
R	11-11-4	-40	-70		FRONT	VERT	TOTAL
T	5-11-4	-40	-70		FRONT	VERT	TOTAL
V	7-11-4	-101	-101		FRONT	VERT	TOTAL
W	9-11-4	-101	-101		FRONT	VERT	TOTAL
X	11-11-4	-101	-101		FRONT	VERT	TOTAL
Y	13-11-4	-101	-101		FRONT	VERT	TOTAL
Z	17-11-12	-101	-101		FRONT	VERT	TOTAL
AA	19-11-12	-101	-101		FRONT	VERT	TOTAL
AB	21-11-12	-101	-101		FRONT	VERT	TOTAL
AC	23-11-12	-101	-101		FRONT	VERT	TOTAL
AD	1-11-4	-48	-85		FRONT	VERT	TOTAL
AE	3-11-4	-48	-85		FRONT	VERT	TOTAL
AF	7-11-4	-40	-70		FRONT	VERT	TOTAL
AG	9-11-4	-40	-70		FRONT	VERT	TOTAL
AH	13-11-4	-40	-70		FRONT	VERT	TOTAL
AI	17-11-12	-40	-70		FRONT	VERT	TOTAL
AJ	21-11-12	-40	-70		FRONT	VERT	TOTAL
AK	23-11-12	-40	-70		FRONT	VERT	TOTAL
AL	27-11-12	-40	-70		FRONT	VERT	TOTAL
AM	29-11-12	-40	-70		FRONT	VERT	TOTAL



DWG NO. TAM 43649-17
 STRUCTURAL
 COMPONENT ONLY



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF

ALL WEBS EXCEPT 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW-m	MT20	5.0	8.0	2.25	3.50
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	8.0	2.25	3.50
I	TMWW-t	MT20	5.0	6.0		
J	TMV+p	MT20	4.0	4.0		
L	BMWW-t	MT20	5.0	8.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMWW-t	MT20	4.0	9.0		
O	BS-t	MT20	3.0	6.0		
P	BMWW-t	MT20	4.0	4.0	2.00	1.75
Q	BMWW-t	MT20	4.0	4.0		
R	BMWW-t	MT20	5.0	8.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	IN-SX	IN-SX
R	1908 0	1908 0	5-8	5-8
L	1908 0	1908 0	HANGER BY OTHERS	MIN. SEAT SIZE: 3-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1548	886 / 0	335 / 0	0 / 0	0 / 0	327 / 0	0 / 0
L	1548	886 / 0	335 / 0	0 / 0	0 / 0	327 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.62 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 26	-84.3 -84.3	0.11 (1)	10.00	C-Q	0 / 192	0.04 (3)
B-C	0 / 14	-84.3 -84.3	0.17 (1)	10.00	Q-D	0 / 295	0.07 (2)
C-D	-2523 / 0	-84.3 -84.3	0.27 (1)	4.11	D-P	0 / 932	0.21 (1)
D-E	-2943 / 0	-84.3 -84.3	0.51 (1)	3.62	P-E	-490 / 0	0.19 (1)
E-F	-2942 / 0	-84.3 -84.3	0.50 (1)	3.62	E-N	-2 / 0	0.00 (1)
F-G	-2942 / 0	-84.3 -84.3	0.50 (1)	3.62	N-G	-489 / 0	0.19 (1)
G-H	-2941 / 0	-84.3 -84.3	0.50 (1)	3.62	H-I	0 / 930	0.21 (1)
H-I	-2523 / 0	-84.3 -84.3	0.27 (1)	4.11	M-H	0 / 295	0.07 (2)
I-J	0 / 14	-84.3 -84.3	0.17 (1)	10.00	M-I	0 / 192	0.04 (3)
J-K	0 / 26	-84.3 -84.3	0.11 (1)	10.00	R-C	-2684 / 0	0.97 (1)
R-B	-249 / 0	0.0 0.0	0.02 (1)	7.61	I-L	-2684 / 0	0.97 (1)
L-J	-249 / 0	0.0 0.0	0.02 (1)	7.61			
R-Q	0 / 2181	-28.0 -28.0	0.62 (2)	10.00			
Q-P	0 / 2245	-28.0 -28.0	0.65 (2)	10.00			
P-O	0 / 2943	-28.0 -28.0	0.56 (1)	10.00			
O-N	0 / 2943	-28.0 -28.0	0.56 (1)	10.00			
N-M	0 / 2245	-28.0 -28.0	0.65 (2)	10.00			
M-L	0 / 2181	-28.0 -28.0	0.62 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 3.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. G/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(5% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.06")

CALCULATED VERT. DEFL.(LL) = L/999 (0.20")

ALLOWABLE DEFL.(TL) = L/360 (1.06")

CALCULATED VERT. DEFL.(TL) = L/999 (0.33")

CSI: TC=0.51 (D-E-1), BC=0.65 (M-N-2), WB=0.97 (I-L-1), SSI=0.21 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg

JSI GRIP = 0.87 (I) (INPUT = 0.90)

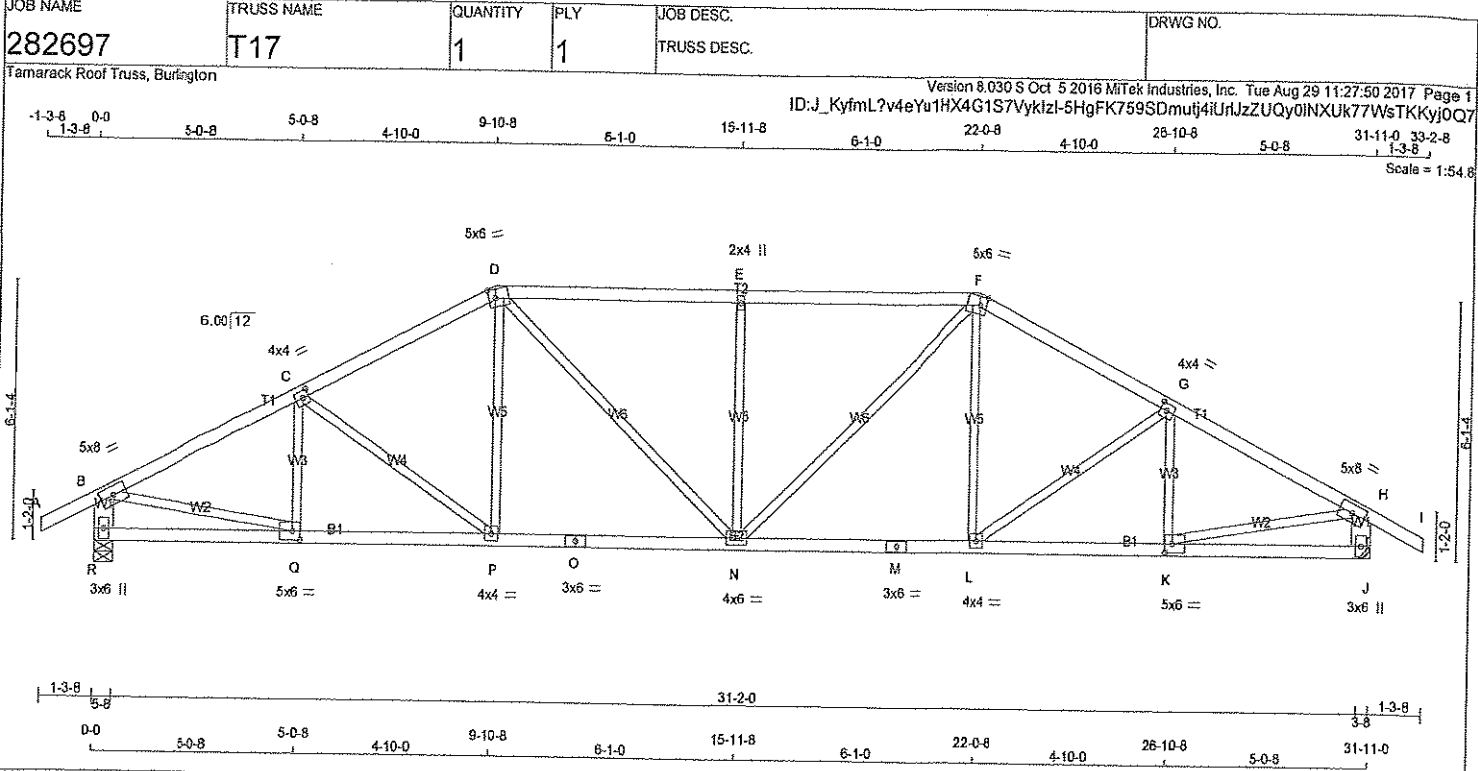
JSI METAL = 0.87 (O) (INPUT = 1.00)



DWG NO. TAM 43650-17

STRUCTURAL

COMPONENT ONLY



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
R - B	2x6	DRY	No.2
J - H	2x6	DRY	No.2
R - O	2x4	DRY	No.2
O - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0		
C	TMVW-t	MT20	4.0	4.0	2.00	1.75
D	TTVW-m	MT20	5.0	5.0	2.50	2.00
E	TMVW-w	MT20	2.0	4.0		
F	TTVW-m	MT20	5.0	6.0	2.50	2.00
G	TMVW-t	MT20	4.0	4.0	2.00	1.75
H	TMVW-t	MT20	5.0	8.0		
J	BMV1+p	MT20	3.0	6.0		
K	BMVW-t	MT20	5.0	6.0	2.50	2.25
L	BMVW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	5.0		
N	BMVW-t	MT20	4.0	6.0		
O	BS-t	MT20	3.0	6.0		
P	BMVW-t	MT20	4.0	4.0		
Q	BMVW-t	MT20	5.0	6.0	2.50	2.25
R	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	UP
JT	1906	0	1906	0
R	1906	0	1906	0
J	1906	0	1906	0

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1548	886/0	335/0	0/0	0/0	327/0	0/0
J	1548	886/0	335/0	0/0	0/0	327/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.85 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LENGTH FR-TO	
A-B	0/26	-84.3	-84.3 0.11 (1)	10.00	Q-C	-255/66	0.05 (1)
B-C	-2585/0	-84.3	-84.3 0.35 (1)	4.01	C-P	-264/6	0.16 (1)
C-D	-2385/0	-84.3	-84.3 0.33 (1)	4.17	P-D	0/364	0.08 (2)
D-E	-2538/0	-84.3	-84.3 0.50 (1)	3.65	D-N	0/577	0.13 (1)
E-F	-2538/0	-84.3	-84.3 0.50 (1)	3.85	N-E	-628/0	0.37 (1)
F-G	-2385/0	-84.3	-84.3 0.33 (1)	4.01	N-F	0/577	0.13 (1)
G-H	-2585/0	-84.3	-84.3 0.35 (1)	4.17	L-F	0/364	0.08 (2)
H-I	0/26	-84.3	-84.3 0.11 (1)	10.00	L-G	-264/0	0.16 (1)
R-B	-1843/0	0.0	0.0 0.12 (1)	7.36	K-G	-255/66	0.08 (1)
J-H	-1843/0	0.0	0.0 0.12 (1)	7.36	B-Q	0/2364	0.53 (1)
R-Q	0/0	-28.0	-28.0 0.15 (2)	10.00	K-H	0/2364	0.53 (1)
Q-P	0/2331	-28.0	-28.0 0.47 (1)	10.00			
P-O	0/2119	-28.0	-28.0 0.40 (2)	10.00			
O-N	0/2119	-28.0	-28.0 0.45 (2)	10.00			
N-M	0/2119	-28.0	-28.0 0.46 (2)	10.00			
M-L	0/2119	-28.0	-28.0 0.46 (2)	10.00			
L-K	0/2331	-28.0	-28.0 0.47 (1)	10.00			
K-J	0/0	-28.0	-28.0 0.15 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 3.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 46.1	PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L^3/360$ (1.06")
CALCULATED VERT. DEFL.(LL) = $L^3/999$ (0.16")
ALLOWABLE DEFL.(TL) = $L^3/360$ (1.06")
CALCULATED VERT. DEFL.(TL) = $L^3/999$ (0.25")

CSI: TC=0.50 (D-E:1), BC=0.47 (P-Q:1), WB=0.53 (B-Q:1), SS=0.25 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

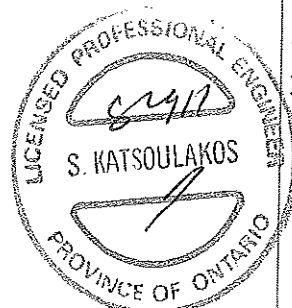
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

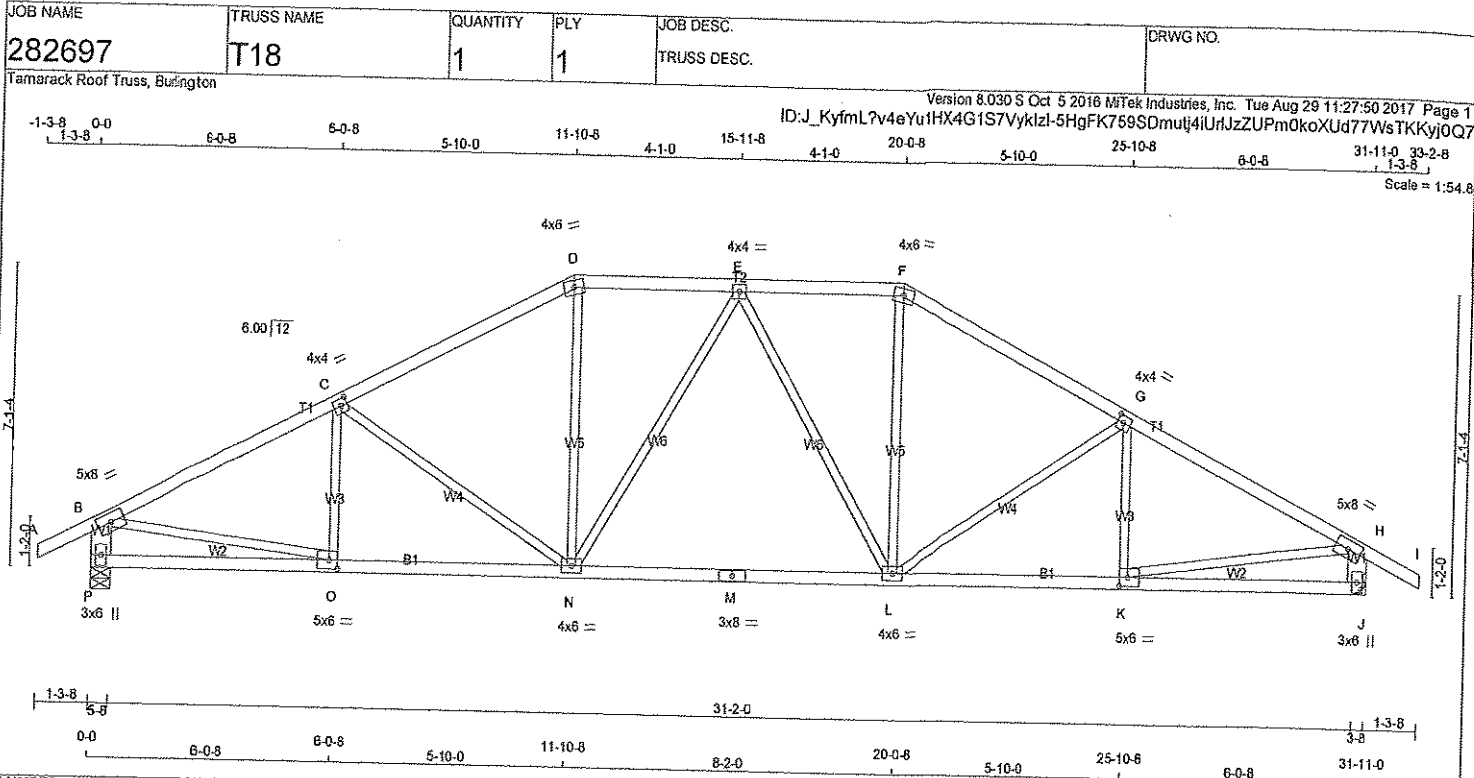
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.87 (D) (INPUT = 0.90)
JSI METAL = 0.81 (O) (INPUT = 1.00)



DWG NO. TAM 43651-17
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
D - F	2x4	DRY No.2	SPF
F - I	2x4	DRY No.2	SPF
P - B	2x6	DRY No.2	SPF
J - H	2x6	DRY No.2	SPF
P - M	2x4	DRY No.2	SPF
M - J	2x4	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-I	MT20	5.0	8.0		
C	TMWW-I	MT20	4.0	4.0	2.00	1.75
D	TTW-m	MT20	4.0	6.0		
E	TMVW-I	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	6.0		
G	TMVW-I	MT20	4.0	4.0	2.00	1.75
H	TMVW-I	MT20	5.0	8.0		
J	BMV1+p	MT20	3.0	6.0		
K	BMVW-I	MT20	5.0	6.0	2.50	2.50
L	BMVW-I	MT20	4.0	6.0		
M	BS-I	MT20	3.0	8.0		
N	BMVW-I	MT20	4.0	6.0		
O	BMVW-I	MT20	5.0	8.0	2.50	2.50
P	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	IN-SX	IN-SX
P	1906	0	5-8	5-8
J	1906	0	0	0

HANGER BY OTHERS
MIN. SEAT SIZE: 3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
P	1548	888 / 0	335 / 0	0 / 0	0 / 0	327 / 0	0 / 0	0 / 0
J	1548	888 / 0	335 / 0	0 / 0	0 / 0	327 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.81 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 26	-84.3 -84.3	0.11 (1)	10.00	O-C	-167 / 140	0.04 (1)
B-C	-2623 / 0	-84.3 -84.3	0.52 (1)	3.81	C-N	-469 / 0	0.45 (1)
C-D	-2237 / 0	-84.3 -84.3	0.46 (1)	4.13	N-D	0 / 667	0.15 (1)
D-E	-1986 / 0	-84.3 -84.3	0.21 (1)	4.61	N-E	-239 / 0	0.31 (1)
E-F	-1986 / 0	-84.3 -84.3	0.21 (1)	4.61	E-L	-239 / 0	0.31 (1)
F-G	-2237 / 0	-84.3 -84.3	0.46 (1)	4.13	L-F	0 / 667	0.15 (1)
G-H	-2623 / 0	-84.3 -84.3	0.52 (1)	3.81	L-G	-469 / 0	0.45 (1)
H-I	0 / 26	-84.3 -84.3	0.11 (1)	10.00	K-G	-167 / 140	0.04 (1)
P-B	-1832 / 0	0.0 0.0	0.12 (1)	7.39	B-O	0 / 2392	0.54 (1)
J-H	-1832 / 0	0.0 0.0	0.12 (1)	7.39	K-H	0 / 2392	0.54 (1)
P-O	0 / 0	-28.0 -28.0	0.21 (2)	10.00			
O-N	0 / 2389	-28.0 -28.0	0.57 (2)	10.00			
N-M	0 / 2107	-28.0 -28.0	0.54 (2)	10.00			
M-L	0 / 2107	-28.0 -28.0	0.54 (2)	10.00			
L-K	0 / 2389	-28.0 -28.0	0.57 (2)	10.00			
K-J	0 / 0	-28.0 -28.0	0.21 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/380 (1.68")
CALCULATED VERT. DEFL.(LL) = L/999 (0.19")
ALLOWABLE DEFL.(TL)= L/380 (1.68")
CALCULATED VERT. DEFL.(TL) = L/999 (0.32")

CSI: TC=0.52 (G-H:1), BC=0.57 (N-O:2).
WB=0.54 (B-O:1), SS=0.22 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

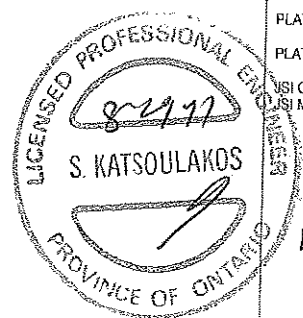
NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667

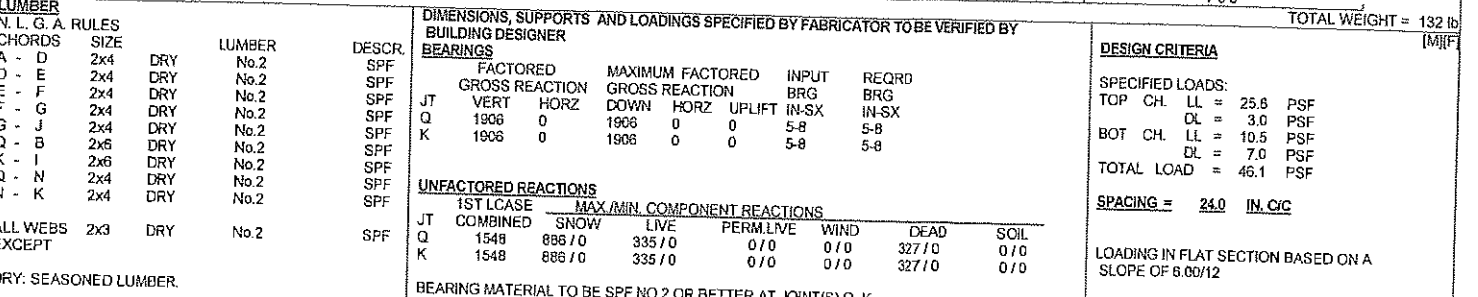
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

ISI GRIP= 0.90 (K) (INPUT = 0.90)
ISI METAL= 0.77 (M) (INPUT = 1.00)



DWG NO. TAM 4365217
STRUCTURAL
COMPONENT ONLY



DIMENSIONS, LOADS, LOADINGS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG			
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX		
Q	1906	0	1906	0	0	5-8	5-8		
K	1906	0	1906	0	0	5-8	5-8		
UNFACTORED REACTIONS									
JT	1ST LCASE COMBINED		MAX MIN. COMPONENT REACTIONS		PERM. LIVE	WIND	DEAD		SOIL
	SNOW	LIVE							
Q	1548	886 / 0	335 / 0		0 / 0	0 / 0	327 / 0		0 / 0
K	1548	886 / 0	335 / 0		0 / 0	0 / 0	327 / 0		0 / 0
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K									
BRACING									
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.53 FT.									
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.									
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.									
1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-O, H-M.									
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW									
LOADING									
TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CS1 (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)		
FR-TO		FROM	TO	10.00	FR-TO				
A-B	0 / 26	-84.3	-84.3 0.11 (1)	10.00	P-C	-67 / 254	0.06 (3)		
B-C	-2638 / 0	-84.3	-84.3 0.74 (1)	3.53	C-O	-689 / 0	0.32 (1)		
C-D	-2048 / 0	-84.3	-84.3 0.63 (1)	4.03	O-E	0 / 509	0.11 (1)		
D-E	-2048 / 0	-84.3	-84.3 0.63 (1)	4.03	E-F	0 / 2	0.00 (1)		
E-F	-1810 / 0	-84.3	-84.3 0.23 (1)	4.77	M-F	0 / 512	0.12 (1)		
F-G	-2049 / 0	-84.3	-84.3 0.63 (1)	4.03	M-H	-668 / 0	0.31 (1)		
G-H	-2049 / 0	-84.3	-84.3 0.63 (1)	4.03	L-H	-69 / 253	0.06 (3)		
H-I	-2638 / 0	-84.3	-84.3 0.74 (1)	3.53	B-P	0 / 2403	0.54 (1)		
I-J	0 / 26	-84.3	-84.3 0.11 (1)	10.00	L-I	0 / 2403	0.54 (1)		
Q-B	-1825 / 0	0.0	0.0 0.12 (1)	7.39					
K-I	-1825 / 0	0.0	0.0 0.12 (1)	7.39					
Q-P	0 / 0	-28.0	-28.0 0.35 (3)	10.00					
P-O	0 / 2386	-28.0	-28.0 0.62 (2)	10.00					
O-N	0 / 1809	-28.0	-28.0 0.39 (1)	10.00					
N-M	0 / 1809	-28.0	-28.0 0.39 (1)	10.00					
M-L	0 / 2386	-28.0	-28.0 0.62 (2)	10.00					
L-K	0 / 0	-28.0	-28.0 0.35 (3)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD	=	46.1	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2012, CBCG 2012, ABC 2014
- CSA C88-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.08")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (1.08")
CALCULATED VERT. DEFL.(TU) = L/ 999 (0.27")

CSI: TC=0.74 (B-C:1), BC=0.62 (O-P:2), WB=0.54 (B-P:1), SSI=0.26 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

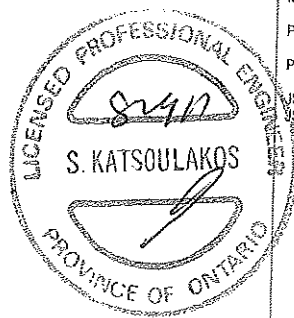
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

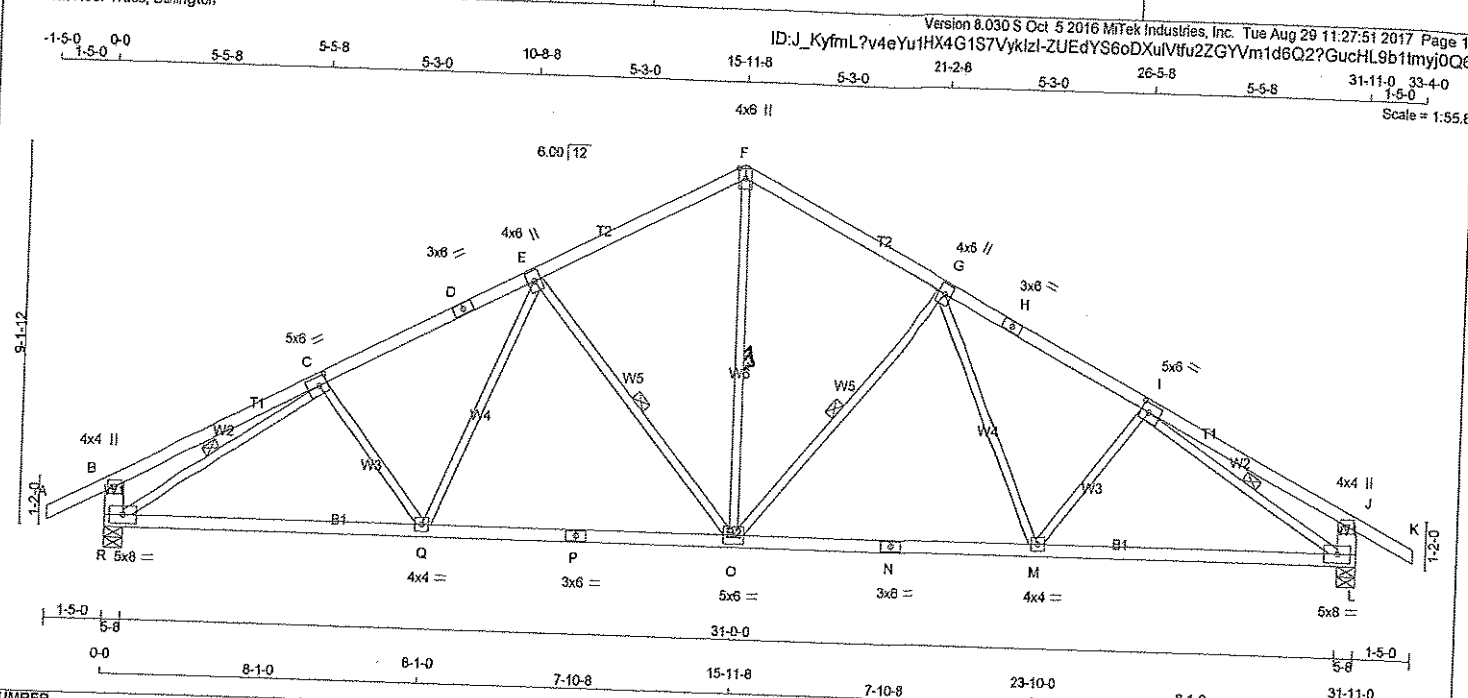
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.88 (P) (INPUT = 0.90)
JSI METAL= 0.60 (P) (INPUT = 1.00)



DWG NO. TAM 43653-17
STRUCTURAL
COMPONENT ONLY



LUMBER

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
R - B	2x6	DRY	No.2
L - J	2x6	DRY	No.2
R - P	2x4	DRY	No.2
P - N	2x4	DRY	No.2
N - L	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2

EXCEPT

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ
JT	1916	0	1916	0
R	1916	0	1916	0
L	1916	0	1916	0

UNFACTORED REACTIONS

	1ST LCASE	MAX/MIN	COMPONENT REACTIONS
	COMBINED	SNOW	LIVE
JT	1558	892 / 0	335 / 0
R	1558	892 / 0	335 / 0
L	1558	892 / 0	335 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 3.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.06")

CALCULATED VERT. DEFL.(LL) = L/999 (0.20")

ALLOWABLE DEFL.(TL) = L/360 (1.06")

CALCULATED VERT. DEFL.(TL) = L/999 (0.33")

CSI: TC=0.35 (C-E:1), BC=0.70 (L-M:2), WB=0.75 (C-R:1), SSI=0.19 (L-M:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMVW-t	MT20	5.0	6.0	2.50	2.50
D	TS-t	MT20	3.0	6.0		
E	TMVW+t	MT20	4.0	6.0		
F	TTW+p	MT20	4.0	6.0		
G	TMVW+t	MT20	4.0	6.0		
H	TS-t	MT20	3.0	6.0		
I	TMVW-t	MT20	5.0	6.0	2.50	2.50
J	TMV+p	MT20	4.0	4.0		
L	BMVW-t	MT20	5.0	6.0		
M	BMVW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	6.0		
O	BMVW-t	MT20	5.0	6.0		
P	BS-t	MT20	3.0	6.0		
Q	BMVW-t	MT20	4.0	4.0		
R	BMVW-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.09 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-O, E-O, C-R, I-L. *P-01*

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

E	-2485 / 0	-84.3	-84.3	0.35 (1)	4.09	M-I	0 / 426	0.10 (2)
F	-1843 / 0	-84.3	-84.3	0.32 (1)	4.64	M-I	-111 / 124	0.04 (1)
G	-1843 / 0	-84.3	-84.3	0.32 (1)	4.64	E-O	-700 / 0	0.31 (1)
H	-2485 / 0	-84.3	-84.3	0.35 (1)	4.09	Q-E	0 / 426	0.10 (2)
I	-2485 / 0	-84.3	-84.3	0.35 (1)	4.09	C-Q	-111 / 124	0.04 (1)
J	0 / 18	-84.3	-84.3	0.35 (1)	4.09	R-C	-2748 / 0	0.75 (1)
K	0 / 28	-84.3	-84.3	0.29 (1)	10.00	I-L	-2748 / 0	0.75 (1)
L	-311 / 0	0.0	0.0	0.02 (1)	7.81			
J	-311 / 0	0.0	0.0	0.02 (1)	7.81			
O	0 / 2292	-28.0	-28.0	0.70 (2)	10.00			
P	0 / 2083	-28.0	-28.0	0.68 (2)	10.00			
O	0 / 2083	-28.0	-28.0	0.68 (2)	10.00			
N	0 / 2083	-28.0	-28.0	0.68 (2)	10.00			
M	0 / 2083	-28.0	-28.0	0.68 (2)	10.00			
L	0 / 2292	-28.0	-28.0	0.70 (2)	10.00			



TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1657
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

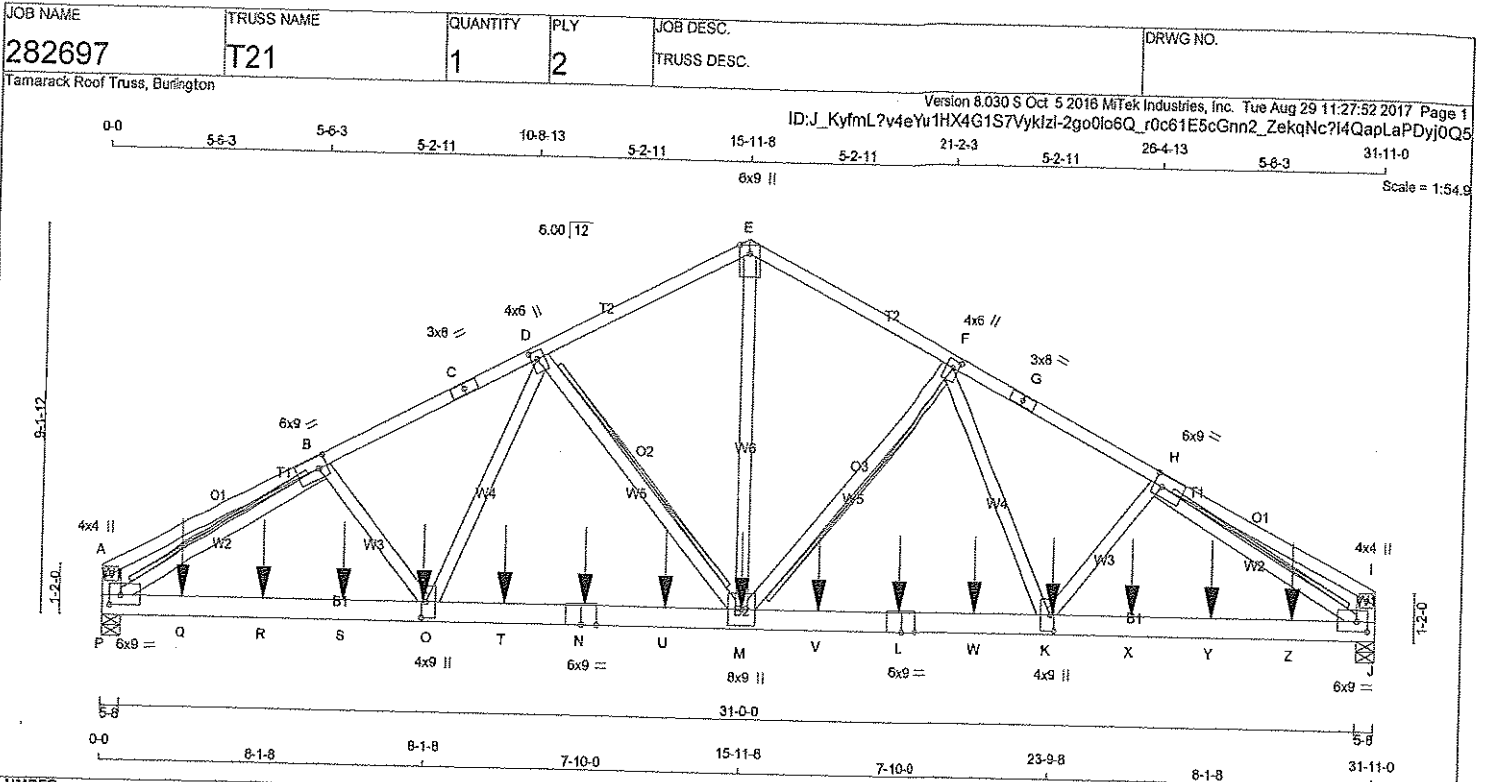
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (C) (INPUT = 0.90)

JSI METAL= 0.68 (I) (INPUT = 1.00)

DWG NO. TAM 43654-17

STRUCTURAL COMPONENT ONLY



LUMBER

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
E - G	2x4	DRY No.2	SPF
G - I	2x4	DRY No.2	SPF
P - A	2x6	DRY No.2	SPF
J - I	2x6	DRY No.2	SPF
P - N	2x6	DRY 2100F 1.8E	SPF
N - L	2x6	DRY 2100F 1.8E	SPF
L - J	2x6	DRY 2100F 1.8E	SPF

ALL WEBS 2x4 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	12	TOP
C - E	12	TOP
E - G	12	TOP
G - I	12	TOP
P - A	2	TOP
J - I	2	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
P - N	2	SIDE(0.0)
N - L	2	SIDE(197.8)
L - J	2	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
M - E	1	SIDE(8.5)
2x4	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	4.0	4.0		
B	TMVW+L	MT20	6.0	9.0	3.25	2.50
C	TS-t	MT20	3.0	8.0		
D	TMVW+L	MT20	4.0	6.0	2.25	1.75
E	TTW+p	MT20	6.0	9.0	Edge	
F	TMVW+L	MT20	4.0	6.0	2.25	1.75
G	TS-t	MT20	3.0	8.0		
H	TMVW+L	MT20	6.0	9.0	3.25	2.50
I	TMV+p	MT20	4.0	4.0		
J	BMVW+L	MT20	6.0	9.0	2.75	3.25
K	BMVW+L	MT20	4.0	9.0	4.50	1.25
L	BS-t	MT20	6.0	9.0		
M	BMVW+L	MT20	6.0	9.0		
N	BS-t	MT20	6.0	9.0		
O	BMVW+L	MT20	4.0	9.0	4.50	1.25
P	BMVW+L	MT20	6.0	9.0	2.75	3.25

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
	VERT	HORZ	UP/LIFT	IN-SX
JT	7689	0	0	5-8
P	7611	0	0	5-8
J	7611	0	0	5-8

UNFACTORED REACTIONS

	1ST CASE	MAX/MIN	COMPONENT REACTIONS
	COMBINED	SNOW	LIVE
JT	6223	3601 / 0	1320 / 0
P	6159	3565 / 0	1306 / 0
J			

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.29 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT F-M, D-M
2x6 DRY SPF No.2 T-BRACE AT B-P, H-J

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO				FR-TO			
A-B	-2 / 13	-84.3	-84.3 0.18 (1)	10.00	M-E	0 / 7238	0.64 (1)
B-C	-11865 / 0	-84.3	-84.3 1.00 (1)	2.29	M-F	-2816 / 0	0.70 (1)
C-D	-11865 / 0	-84.3	-84.3 1.00 (1)	2.29	F-K	0 / 3220	0.28 (1)
D-E	-8512 / 0	-84.3	-84.3 0.58 (1)	3.05	K-H	0 / 576	0.08 (1)
E-F	-8512 / 0	-84.3	-84.3 0.58 (1)	3.05	D-M	-2687 / 0	0.72 (1)
F-G	-11768 / 0	-84.3	-84.3 0.98 (1)	2.31	O-D	0 / 3326	0.29 (1)
G-H	-11768 / 0	-84.3	-84.3 0.98 (1)	2.31	B-O	0 / 987	0.08 (1)
H-I	-2 / 13	-84.3	-84.3 0.18 (1)	10.00	P-B	-11965 / 0	0.93 (1)
P-A	-199 / 0	0.0	0.0 0.01 (1)	7.81	H-J	-11869 / 0	0.92 (1)
J-I	-199 / 0	0.0	0.0 0.01 (1)	7.81			

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
P-Q	0 / 10062	-28.0	-28.0 0.74 (1)	10.00		
Q-R	0 / 10062	-28.0	-28.0 0.74 (1)	10.00		
R-S	0 / 10062	-28.0	-28.0 0.74 (1)	10.00		
S-O	0 / 10062	-28.0	-28.0 0.74 (1)	10.00		
O-T	0 / 9332	-28.0	-28.0 0.72 (1)	10.00		
T-N	0 / 9332	-28.0	-28.0 0.72 (1)	10.00		
N-U	0 / 9332	-28.0	-28.0 0.72 (1)	10.00		
U-M	0 / 9332	-28.0	-28.0 0.72 (1)	10.00		
M-V	0 / 9286	-28.0	-28.0 0.71 (1)	10.00		
V-L	0 / 9286	-28.0	-28.0 0.71 (1)	10.00		
L-W	0 / 9286	-28.0	-28.0 0.71 (1)	10.00		
W-K	0 / 9286	-28.0	-28.0 0.71 (1)	10.00		
K-X	0 / 9982	-28.0	-28.0 0.72 (1)	10.00		
X-Y	0 / 9982	-28.0	-28.0 0.72 (1)	10.00		
Y-Z	0 / 9982	-28.0	-28.0 0.72 (1)	10.00		
Z-J	0 / 9982	-28.0	-28.0 0.72 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (1.06")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.29")
ALLOWABLE DEFL.(TL) = $L/900$ (1.06")
CALCULATED VERT. DEFL.(TL) = $L/828$ (0.45")

CSI: TC=1.00 (B-D-1), BC=0.74 (O-P-1), WB=0.93 (B-P-1), SSI=0.53 (O-P-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (P) (INPUT = 0.50)
JSI METAL= 0.99 (H) (INPUT = 1.00)



DRWG NO. TAM 43655-17
STRUCTURAL
COMPONENT ONLY

Tamarack Roof Truss, Burlington

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

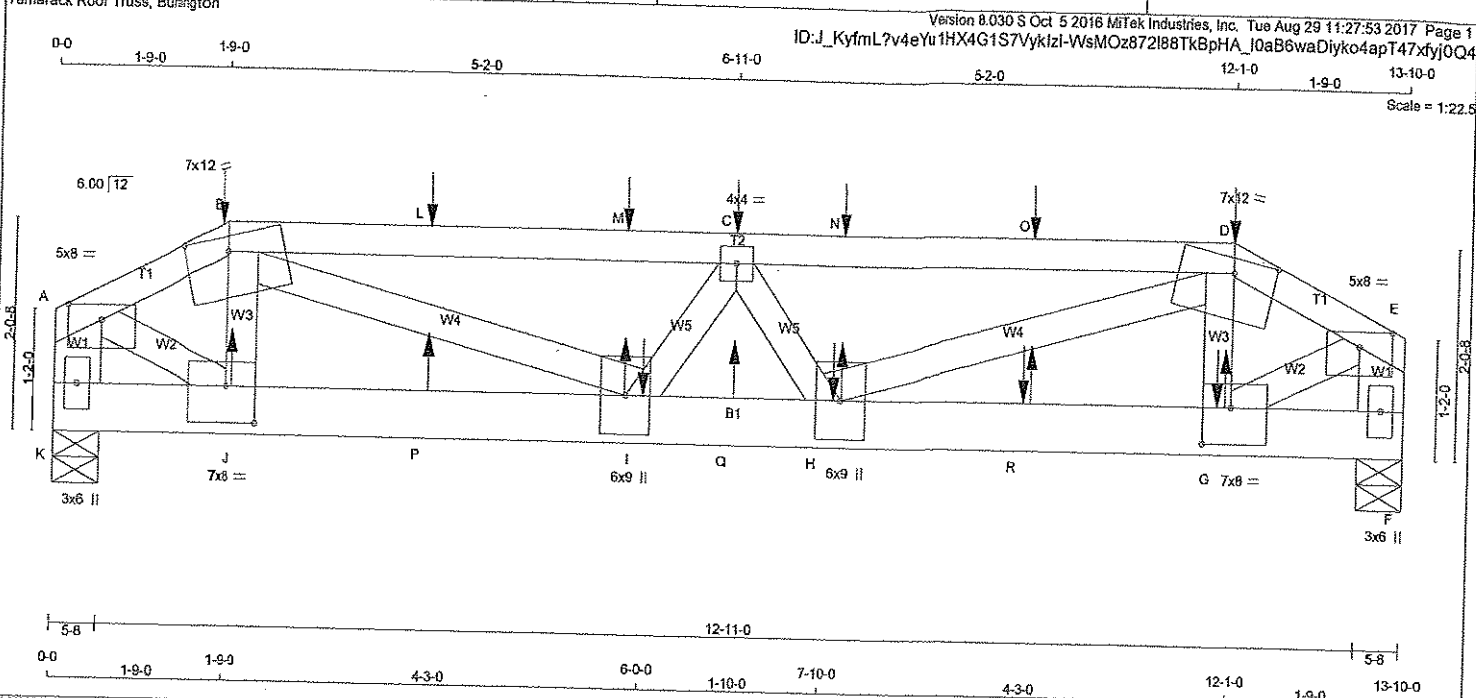
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 790.5 lbs FACTORED DOWN AT 2-0-12, 790.5 lbs FACTORED DOWN AT 4-0-12, 790.5 lbs FACTORED DOWN AT 6-0-12, 790.5 lbs FACTORED DOWN AT 8-0-12, 790.5 lbs FACTORED DOWN AT 10-0-12, 790.5 lbs FACTORED DOWN AT 12-0-12, 790.5 lbs FACTORED DOWN AT 14-0-12, 790.5 lbs FACTORED DOWN AT 15-11-8, 790.5 lbs FACTORED DOWN AT 17-10-4, 767.1 lbs FACTORED DOWN AT 19-10-4, 767.1 lbs FACTORED DOWN AT 21-10-4, 767.1 lbs FACTORED DOWN AT 23-10-4, 767.1 lbs FACTORED DOWN AT 25-10-4, AND 767.1 lbs FACTORED DOWN AT 27-10-4, AND 767.1 lbs FACTORED DOWN AT 29-10-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
K	23-10-4	-767	-767	---	BACK	VERT	TOTAL
L	19-10-4	-767	-767	---	BACK	VERT	TOTAL
M	15-11-8	-791	-791	---	BACK	VERT	TOTAL
N	12-0-12	-791	-791	---	BACK	VERT	TOTAL
O	8-0-12	-791	-791	---	BACK	VERT	TOTAL
Q	2-0-12	-791	-791	---	BACK	VERT	TOTAL
R	4-0-12	-791	-791	---	BACK	VERT	TOTAL
S	6-0-12	-791	-791	---	BACK	VERT	TOTAL
T	10-0-12	-791	-791	---	BACK	VERT	TOTAL
U	14-0-12	-791	-791	---	BACK	VERT	TOTAL
V	17-10-4	-791	-791	---	BACK	VERT	TOTAL
W	21-10-4	-767	-767	---	BACK	VERT	TOTAL
X	25-10-4	-767	-767	---	BACK	VERT	TOTAL
Y	27-10-4	-767	-767	---	BACK	VERT	TOTAL
Z	29-10-4	-767	-767	---	BACK	VERT	TOTAL



DWG NO. TAM43655-17
 STRUCTURAL
 COMPONENT ONLY



LUMBER

CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY 1650F 1.5E	SPF
B - D	2x4	DRY 1650F 1.5E	SPF
D - E	2x4	DRY 1650F 1.5E	SPF
K - A	2x6	DRY No.2	SPF
F - E	2x6	DRY No.2	SPF
K - F	2x6	DRY 1650F 1.5E	SPF

ALL WEBS 2x4 DRY No.2 SPF EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - B	12	SIDE(61.0)
B - D	12	SIDE(61.0)
D - E	12	SIDE(61.0)
K - A	2	TOP
F - E	2	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
K - F	12	SIDE(183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
J - B	2	SIDE(27.0)
G - D	2	SIDE(489.7)
2x4	1	6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	8.0	1.75	4.00
B	TTWW-m	MT20	7.0	12.0	1.75	5.25
C	TMVW-i	MT20	4.0	4.0		
D	TTWW-m	MT20	7.0	12.0	1.75	5.25
E	TMVW-p	MT20	5.0	8.0	1.75	4.00
F	BMV1+p	MT20	3.0	6.0		
G	BMVW-L	MT20	7.0	8.0	4.25	3.50
H	BMVW-H	MT20	6.0	9.0		
I	BMVW-H	MT20	6.0	9.0		
J	BMVW-L	MT20	7.0	8.0	4.25	3.50
K	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES
1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	FACTORED GROSS REACTION HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	MAXIMUM FACTORED GROSS REACTION HORZ	INPUT BRG UPLIFT IN-SX	REQD BRG IN-SX
K	3943	0	3943	0	5-8	5-8
F	6003	0	6003	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. SNOW	MAX/MIN. LIVE	MAX/MIN. PERM. LIVE	MAX/MIN. WIND	MAX/MIN. DEAD	MAX/MIN. SOIL
K	3227	1808 / 0	723 / 0	0 / 0	0 / 0	695 / 0	0 / 0
F	4880	2788 / 0	1059 / 0	0 / 0	0 / 0	1033 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

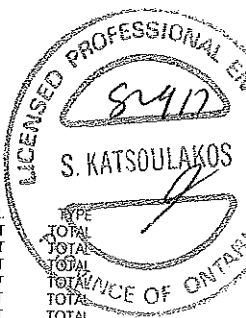
LOADING
TOTAL LOAD CASES: (7)

CHORDS	MAX. FACTORED MEMB. FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED CSI (LC)	MAX. FACTORED UNBRAC LENGTH	WEBS	MAX. FACTORED MEMB. FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO					FR-TO		
A-B	-5045 / 0	-84.3	0.05 (1)	4.85	J-B	-1115 / 0	0.05 (1)
B-L	-12300 / 0	-84.3	0.49 (1)	3.01	B-I	0 / 8474	0.75 (1)
L-M	-12300 / 0	-84.3	0.49 (1)	3.01	I-C	-1285 / 0	0.08 (1)
M-C	-12300 / 0	-84.3	0.49 (1)	3.01	C-H	0 / 814	0.07 (1)
C-N	-13312 / 0	-84.3	0.54 (1)	2.86	H-D	0 / 6722	0.59 (1)
N-O	-13312 / 0	-84.3	0.54 (1)	2.86	G-D	0 / 779	0.07 (1)
O-D	-13312 / 0	-84.3	0.54 (1)	2.86	A-J	0 / 4864	0.43 (1)
D-E	-7821 / 0	-84.3	0.10 (1)	4.01	G-E	0 / 7542	0.07 (1)
K-A	-4187 / 0	0.0	0.0 0.15 (1)	6.98			
F-E	-6451 / 0	0.0	0.0 0.23 (1)	5.85			

JT	LOC.	LC1	MAX.	FACE	DIR.
K-J	0 / 0	-28.0	-28.0 0.13 (1)	10.00	
J-P	0 / 4405	-28.0	-28.0 0.45 (1)	10.00	
P-I	0 / 4405	-28.0	-28.0 0.45 (1)	10.00	
I-Q	0 / 12920	-28.0	-28.0 0.80 (1)	10.00	
Q-H	0 / 12920	-28.0	-28.0 0.80 (1)	10.00	
H-R	0 / 7050	-28.0	-28.0 0.68 (1)	10.00	
R-G	0 / 7050	-28.0	-28.0 0.68 (1)	10.00	
G-F	0 / 0	-28.0	-28.0 0.23 (1)	10.00	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.
B	1-9-0	-1	-2	104	FRONT VERT
C	6-11-0	-1	-2	104	FRONT VERT
D	12-1-0	-1	-2	104	FRONT VERT
G	11-11-4	-1878	-1878	8	BACK VERT
G	12-0-4	8	8	8	FRONT VERT
H	7-11-4	-1878	-1878	8	BACK VERT
H	8-0-4	8	8	8	FRONT VERT
I	5-9-12	8	8	8	FRONT VERT
I	6-0-0	-2808	-2808	8	BACK VERT
J	1-9-12	8	8	8	FRONT VERT
L	3-9-12	-1	-2	104	FRONT VERT
M	5-9-12	-1	-2	104	FRONT VERT
N	8-0-4	-1	-2	104	FRONT VERT
O	10-0-4	-1	-2	104	FRONT VERT
P	3-9-12	8	8	8	FRONT VERT
Q	6-11-0	8	8	8	FRONT VERT
R	9-11-4	-1878	-1878	8	BACK VERT
R	10-0-4	8	8	8	FRONT VERT



DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 085-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.46")
CALCULATED VERT. DEFL.(LL) = L/920 (0.18")
ALLOWABLE DEFL.(TL) = L/360 (0.46")
CALCULATED VERT. DEFL.(TL) = L/577 (0.29")

CSI: TC=0.54 (C-D:1), BC=0.60 (H-I:1), WB=0.75 (B-I:1), SS=0.42 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (G) (INPUT = 0.90)
JSI METAL= 0.92 (I) (INPUT = 1.00)

DRWG NO. TAM 43656-17
STRUCTURAL
COMPONENT ONLY

JOB NAME 282697	TRUSS NAME T22	QUANTITY 1	PLY 2	JOB DESC. TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

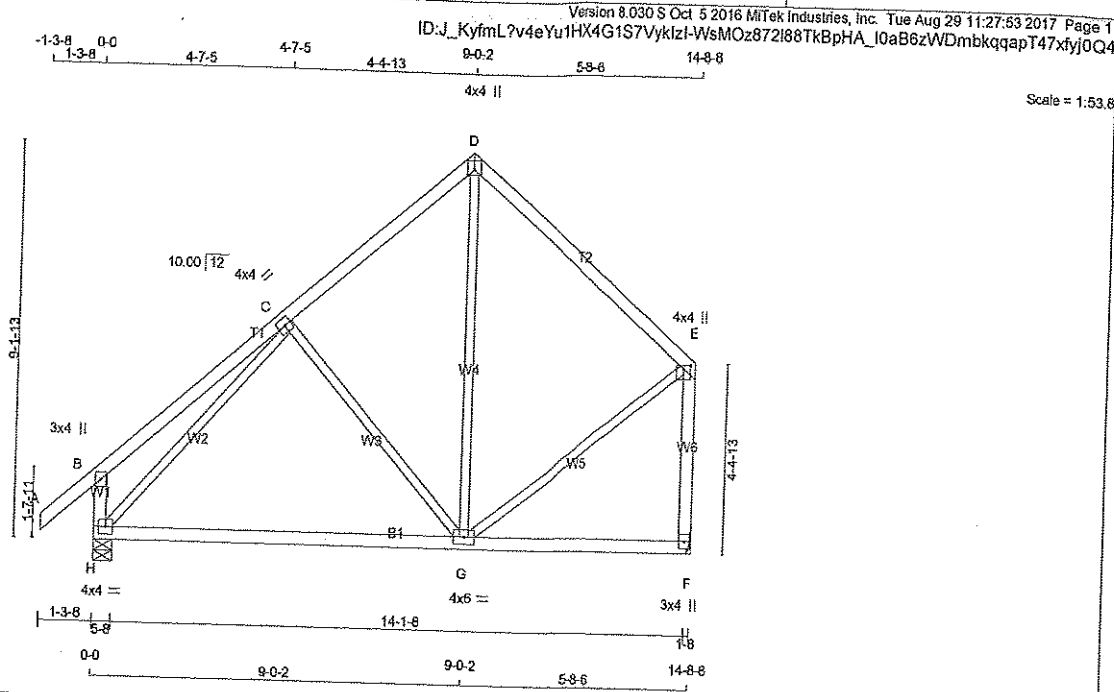
Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Aug 29 11:27:53 2017 Page 2
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HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 2.4 lbs FACTORED DOWN AND 104.0 lbs FACTORED UP AT 1-9-0, 1.9 lbs FACTORED DOWN AND 104.4 lbs FACTORED UP AT 3-9-12, 1.9 lbs FACTORED DOWN AND 104.4 lbs FACTORED UP AT 5-9-12, 1.9 lbs FACTORED DOWN AND 104.4 lbs FACTORED UP AT 6-11-0, 1.9 lbs FACTORED DOWN AND 104.4 lbs FACTORED UP AT 8-0-4, AND 1.9 lbs FACTORED DOWN AND 104.4 lbs FACTORED UP AT 10-0-4, AND 2.4 lbs FACTORED DOWN AND 104.0 lbs FACTORED UP AT 12-1-0 ON TOP CHORD, AND 7.6 lbs FACTORED UP AT 1-9-12, 7.6 lbs FACTORED UP AT 3-9-12, 7.6 lbs FACTORED UP AT 5-9-12, 2807.7 lbs FACTORED DOWN AT 6-0-0, 7.6 lbs FACTORED UP AT 6-11-0, 1877.7 lbs FACTORED DOWN AT 7-11-4, 7.6 lbs FACTORED UP AT 8-0-4, 1877.7 lbs FACTORED DOWN AT 9-11-4, 7.6 lbs FACTORED UP AT 10-0-4, AND 1877.7 lbs FACTORED DOWN AT 11-11-4, AND 7.6 lbs FACTORED UP AT 12-0-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.



DWG NO. TAM 43656-17
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW+p	MT20	4.0	4.0	1.50	2.00
E	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMVWW-t	MT20	4.0	6.0		
H	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
F	826	0	826	0	IN-SX	IN-SX
H	942	0	942	0	MIN. SEAT SIZE: 1-8	5-8

UNFACTORED REACTIONS

JT	1ST LC CASE COMBINED		MAX/MIN COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	WIND			
F	678	376 / 0	154 / 0	0 / 0	0 / 0	147 / 0	0 / 0
H	757	447 / 0	154 / 0	0 / 0	0 / 0	155 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING									
TOTAL LOAD CASES: (4)									
CHORDS						WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LOAD LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO				FR-TO			
A-B	0 / 37	-84.3	-84.3	0.12 (1)	10.00	C-G	-246 / 13	0.21 (1)	
B-C	0 / 29	-84.3	-84.3	0.28 (1)	10.00	G-D	0 / 318	0.07 (2)	
C-D	-486 / 0	-84.3	-84.3	0.22 (1)	6.25	H-C	-776 / 0	0.64 (1)	
D-E	-457 / 0	-84.3	-84.3	0.35 (1)	6.25	G-E	0 / 431	0.10 (1)	
H-B	-259 / 0	0.0	0.0	0.03 (1)	7.81				
F-E	-783 / 0	0.0	0.0	0.26 (1)	7.81				
H-G	0 / 516	-28.0	-28.0	0.57 (2)	10.00				
G-F	0 / 0	-28.0	-28.0	0.51 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 3.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 45.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.49")

CALCULATED VERT. DEFL. (LL) = $L/740$ (0.24")

ALLOWABLE DEFL. (TL) = $L/360$ (0.49")

CALCULATED VERT. DEFL. (TL) = $L/445$ (0.40")

CSI: TC=0.35 (D-E:1), BC=0.57 (G-H:2), WB=0.64 (C-H:1), SSI=0.21 (G-H:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

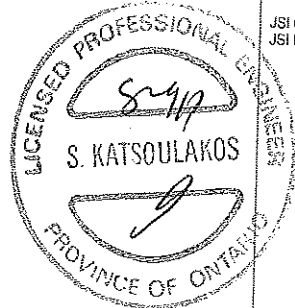
PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1687
	822	2284	1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (D) (INPUT = 0.90)

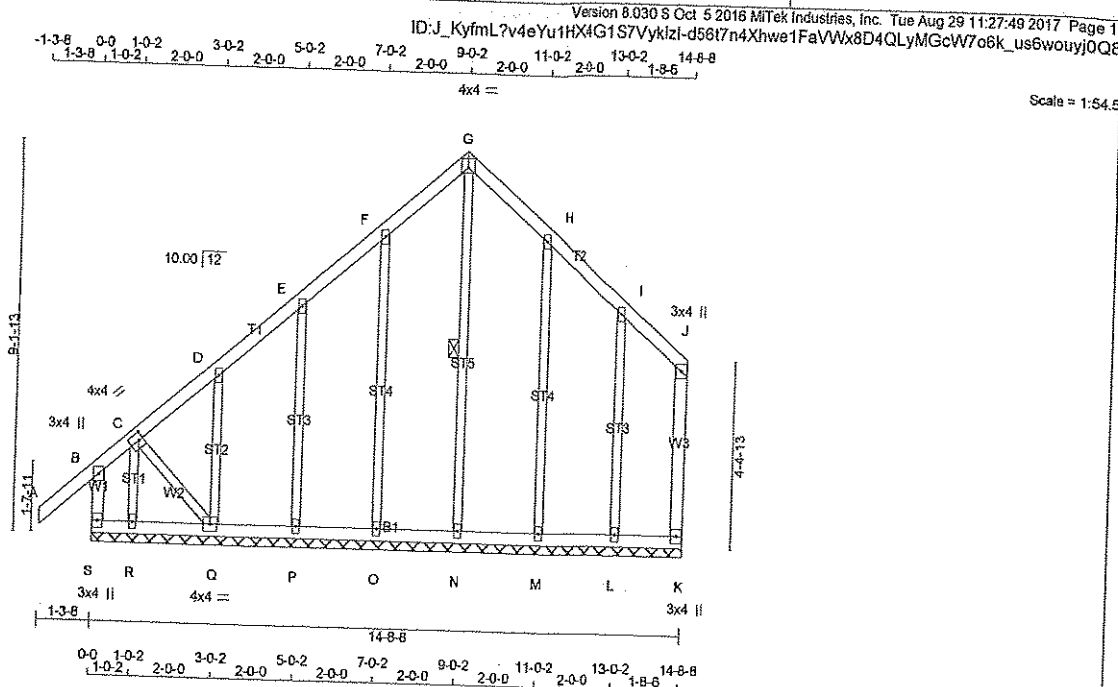
JSI METAL= 0.28 (C) (INPUT = 1.00)



DRWG NO. TAM 43657-17

STRUCTURAL

COMPONENT ONLY



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
S - K	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D, E, F, H, I						
D	TMW+w	MT20	2.0	4.0		
G	TTW-p	MT20	4.0	4.0	1.50	2.00
J	TMV+p	MT20	3.0	4.0		
K	BMV1+p	MT20	3.0	4.0		
L, M, N, O, P, R						
L	BMW1+w	MT20	2.0	4.0		
Q	BMWW1-t	MT20	4.0	4.0		
S	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.
1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-N.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	MEMB.
FR-TO		FROM TO		FR-TO			
A-B	0/37	-84.3 -84.3 0.12 (1)	10.00	N-G	-150/0	0.08 (1)	
B-C	-46/0	-84.3 -84.3 0.11 (1)	6.25	O-F	-193/0	0.19 (1)	
C-D	0/2	-84.3 -84.3 0.04 (1)	10.00	P-E	-159/0	0.08 (1)	
D-E	-1/0	-84.3 -84.3 0.04 (1)	10.00	Q-D	-175/0	0.05 (1)	
E-F	0/7	-84.3 -84.3 0.05 (1)	10.00	R-C	-48/0	0.01 (1)	
F-G	-4/0	-84.3 -84.3 0.05 (1)	10.00	M-H	-194/0	0.20 (1)	
G-H	-4/0	-84.3 -84.3 0.05 (1)	10.00	L-I	-158/0	0.08 (1)	
H-I	0/8	-84.3 -84.3 0.05 (1)	10.00	C-Q	0/6	0.00 (1)	
I-J	0/10	-84.3 -84.3 0.03 (1)	7.81				
S-B	-231/0	0.0 0.0 0.02 (1)	7.81				
K-J	-53/0	0.0 0.0 0.02 (1)	7.81				
S-R	0/0	-28.0 -28.0 0.02 (2)	10.00				
R-Q	0/0	-28.0 -28.0 0.02 (2)	10.00				
Q-P	0/0	-28.0 -28.0 0.02 (2)	10.00				
P-O	-3/0	-28.0 -28.0 0.02 (2)	10.00				
O-N	-5/0	-28.0 -28.0 0.02 (2)	10.00				
N-M	-5/0	-28.0 -28.0 0.02 (2)	10.00				
M-L	-3/0	-28.0 -28.0 0.02 (2)	10.00				
L-K	0/0	-28.0 -28.0 0.02 (2)	10.00				

TOTAL WEIGHT = 80 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN./C/C
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.12 (A-B:1), BC=0.02 (P-Q:2), WB=0.20 (H-M:1), SSI=0.07 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

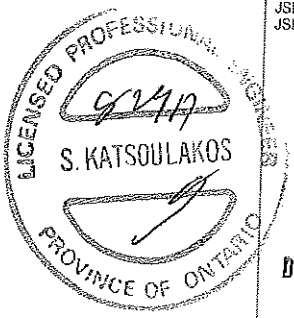
PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

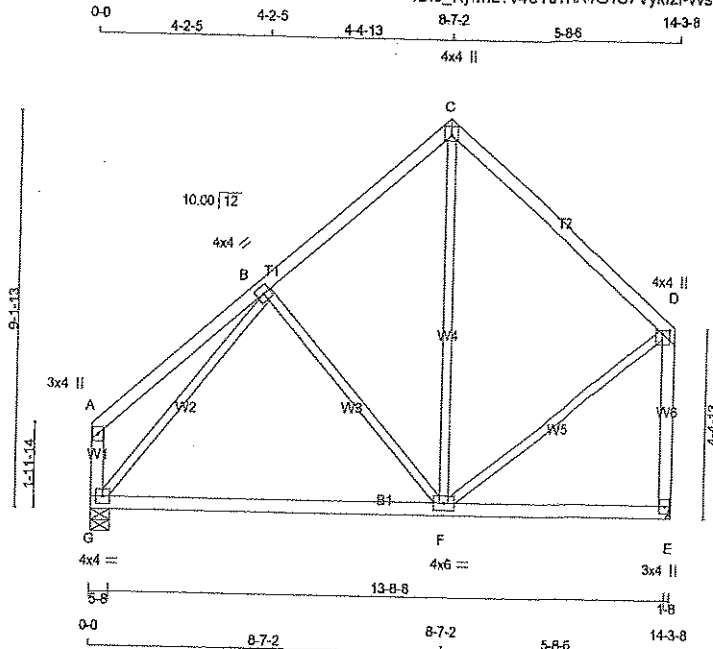
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (G) (INPUT = 0.90)

JSI METAL= 0.05 (H) (INPUT = 1.00)



DRWG NO. TAM 4366/17
STRUCTURAL
COMPONENT ONLY



Scale = 1:53.6

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	DESCR.
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
G - A	2x4	DRY	No.2
E - D	2x4	DRY	No.2
G - E	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	4.0		
C	TTW+p	MT20	4.0	4.0	1.50	2.00
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMV1+p	MT20	3.0	4.0		
F	BMWW-t	MT20	4.0	6.0		
G	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
	VERT	HORZ	IN-SX	IN-SX
E	802	0	0	0
G	802	0	0	0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX. SNOW	MIN. LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	659	388 / 0	150 / 0	0 / 0	0 / 0	143 / 0	0 / 0
G	659	388 / 0	150 / 0	0 / 0	0 / 0	143 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 30	-84.3 -84.3	0.26 (1)	10.00	B-F	-200 / 26	0.17 (1)
B-C	-461 / 0	-84.3 -84.3	0.20 (1)	6.25	F-C	0 / 292	0.07 (2)
C-D	-435 / 0	-84.3 -84.3	0.35 (1)	6.25	G-B	-745 / 0	0.55 (1)
G-A	-125 / 0	0.0 0.0	0.01 (1)	7.81	F-D	0 / 410	0.09 (1)
E-D	-757 / 0	0.0 0.0	0.25 (1)	7.81			
G-F	0 / 467	-28.0 -28.0	0.52 (2)	10.00			
F-E	0 / 0	-28.0 -28.0	0.47 (3)	10.00			

TOTAL WEIGHT = 6 X 67 = 401 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 3.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 48.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.48")
CALCULATED VERT. DEFL. (LL) = $L/875$ (0.20")
ALLOWABLE DEFL. (TL) = $L/360$ (0.48")
CALCULATED VERT. DEFL. (TL) = $L/526$ (0.33")

CSI: TC=0.35 (C-D:1), BC=0.52 (F-G:2), WB=0.55 (B-G:1), SSI=0.20 (F-G:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

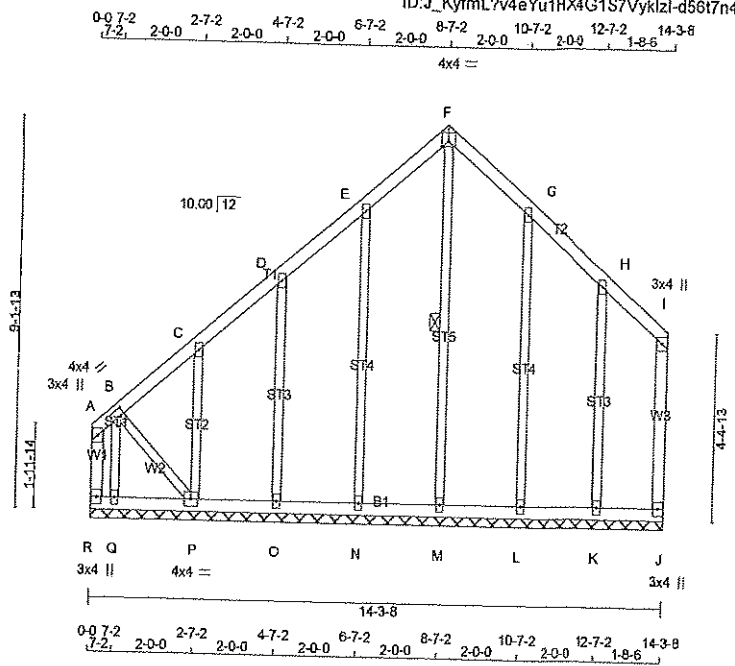
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (B) (INPUT = 0.90)
JSI METAL= 0.27 (B) (INPUT = 1.00)



DWG NO. TAM 4365017
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - A	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
R - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C, D, E, G, H						
C	TMW+w	MT20	2.0	4.0		
F	TTW-p	MT20	4.0	4.0	1.50	2.00
I	TMV+p	MT20	3.0	4.0		
J	BMV1+p	MT20	3.0	4.0		
K, L, M, N, O, Q						
K	BMW1+w	MT20	2.0	4.0		
P	BMWW1-t	MT20	4.0	4.0		
R	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

PROVIDE ANCHORAGE AT BEARING JOINT R FOR 150 LBS. FACTORED UPLIFT BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S).

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.
1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	MAX. UNBRACED LENGTH (FT)
FR-TO		FROM TO		FR-TO			
A-B	0/17	-84.3 -84.3	0.02 (1)	10.00	M-F	-150/0	0.08 (1)
B-C	-1/0	-84.3 -84.3	0.04 (1)	10.00	N-E	-182/0	0.19 (1)
C-D	0/0	-84.3 -84.3	0.04 (1)	10.00	O-D	-160/0	0.08 (1)
D-E	0/7	-84.3 -84.3	0.05 (1)	10.00	P-C	-169/0	0.04 (1)
E-F	-4/0	-84.3 -84.3	0.05 (1)	10.00	Q-B	-134/0	0.02 (1)
F-G	-4/0	-84.3 -84.3	0.05 (1)	10.00	L-G	-194/0	0.20 (1)
G-H	0/8	-84.3 -84.3	0.05 (1)	10.00	K-H	-159/0	0.08 (1)
H-I	0/10	-84.3 -84.3	0.03 (1)	10.00	B-P	0/6	0.00 (1)
R-A	0/2	0.0 0.0	0.00 (1)	10.00			
J-I	-53/0	0.0 0.0	0.02 (1)	7.81			
R-Q	0/0	-28.0 -28.0	0.02 (2)	10.00			
Q-P	0/0	-28.0 -28.0	0.02 (2)	10.00			
P-O	0/0	-28.0 -28.0	0.02 (2)	10.00			
O-N	-3/0	-28.0 -28.0	0.02 (2)	10.00			
N-M	-5/0	-28.0 -28.0	0.02 (2)	10.00			
M-L	-5/0	-28.0 -28.0	0.02 (2)	10.00			
L-K	-3/0	-28.0 -28.0	0.02 (2)	10.00			
K-J	0/0	-28.0 -28.0	0.02 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.05 (G-H:1), BC=0.02 (P-Q:2), WB=0.20 (G-L:1), SSI=0.06 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1655

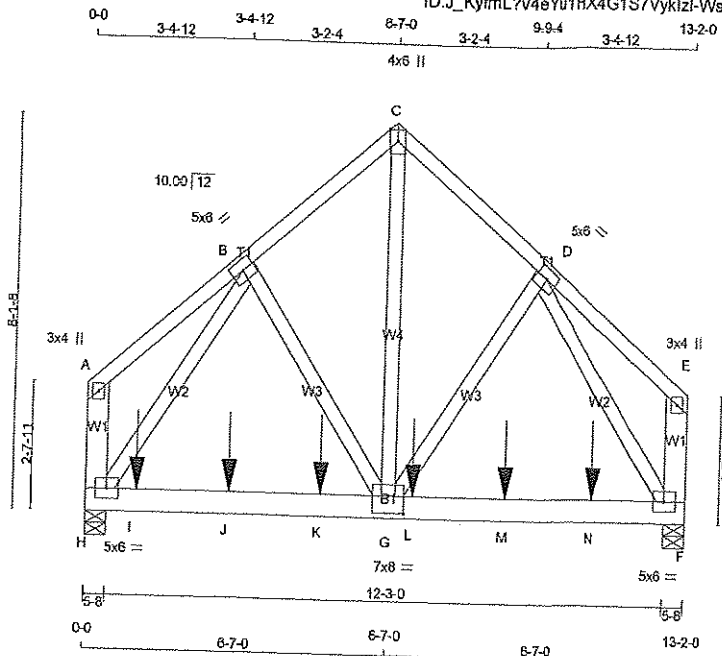
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (F) (INPUT = 0.90)
JSI METAL= 0.05 (G) (INPUT = 1.00)



DRWG NO. TAM 4362-17
STRUCTURAL
COMPONENT ONLY



LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - A	2x6	DRY	No.2	SPF
E - F	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	12	TOP
C - E	12	TOP
H - A	12	TOP
F - E	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
H - F	12	SIDE (0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x4	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
A	TMV+p	MT20	3.0	4.0
B	TMVW-I	MT20	5.0	6.0
C	TTW+p	MT20	4.0	6.0
D	TMVW-I	MT20	5.0	6.0
E	TMV+p	MT20	3.0	4.0
F	BMVW-I	MT20	5.0	6.0
G	BMVW-I	MT20	7.0	8.0
H	BMVW-I	MT20	5.0	6.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 767.1 lbs FACTORED DOWN AT 1-1-4, 767.1 lbs FACTORED DOWN AT 3-1-4, 767.1 lbs FACTORED DOWN AT 5-1-4, 767.1 lbs FACTORED DOWN AT 7-1-4, AND 767.1 lbs FACTORED DOWN AT 9-1-4, AND 767.1 lbs FACTORED DOWN AT 11-1-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQD	
GROSS REACTION		GROSS REACTION		BRG		BRG	
JT	VERT	JT	HORZ	JT	IN-SX	JT	IN-SX
H	3208	0	3208	0	5-8	5-8	
F	2873	0	2873	0	5-8	5-8	

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	2595	1503 / 0	549 / 0	0 / 0	0 / 0	543 / 0	0 / 0
F	2325	1345 / 0	494 / 0	0 / 0	0 / 0	487 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.95 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	-1 / 16	-84.3 -84.3	0.08 (1)	10.00	G-C	0 / 2538	0.22 (1)
B-C	-2180 / 0	-84.3 -84.3	0.08 (1)	5.95	G-D	0 / 484	0.04 (1)
C-D	-2180 / 0	-84.3 -84.3	0.08 (1)	5.95	B-G	0 / 484	0.04 (1)
D-E	-1 / 16	-84.3 -84.3	0.08 (1)	10.00	H-B	-2529 / 0	0.50 (1)
H-A	-111 / 0	0.0 0.0	0.01 (1)	7.81	D-F	-2529 / 0	0.50 (1)
F-E	-111 / 0	0.0 0.0	0.01 (1)	7.81			
H-I	0 / 1390	-28.0 -28.0	0.88 (1)	10.00			
I-J	0 / 1390	-28.0 -28.0	0.88 (1)	10.00			
J-K	0 / 1390	-28.0 -28.0	0.88 (1)	10.00			
K-L	0 / 1390	-28.0 -28.0	0.88 (1)	10.00			
L-M	0 / 1390	-28.0 -28.0	0.88 (1)	10.00			
M-N	0 / 1390	-28.0 -28.0	0.88 (1)	10.00			
N-F	0 / 1390	-28.0 -28.0	0.88 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)				
JT	LOC.	LC1	MAX.	FACE
I	1-1-4	-767	-767	FRONT
J	3-1-4	-767	-767	FRONT
K	5-1-4	-767	-767	FRONT
L	7-1-4	-767	-767	FRONT
M	9-1-4	-767	-767	FRONT
N	11-1-4	-767	-767	FRONT

TOTAL WEIGHT = 2 X 85 = 169 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 3.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.44")

CALCULATED VERT. DEFL.(LL) = $L/999$ (0.09")

ALLOWABLE DEFL.(TL) = $L/360$ (0.44")

CALCULATED VERT. DEFL.(TL) = $L/999$ (0.14")

CSI: TC=0.08 (B-C:1), BC=0.88 (F-G:1), WB=0.50 (D-F:1), SSI=0.61 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00

COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

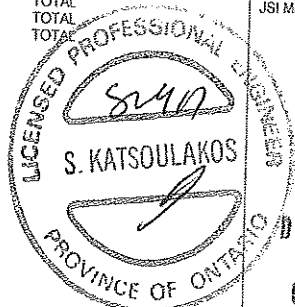
NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	618	354	1657
	622	2284	1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

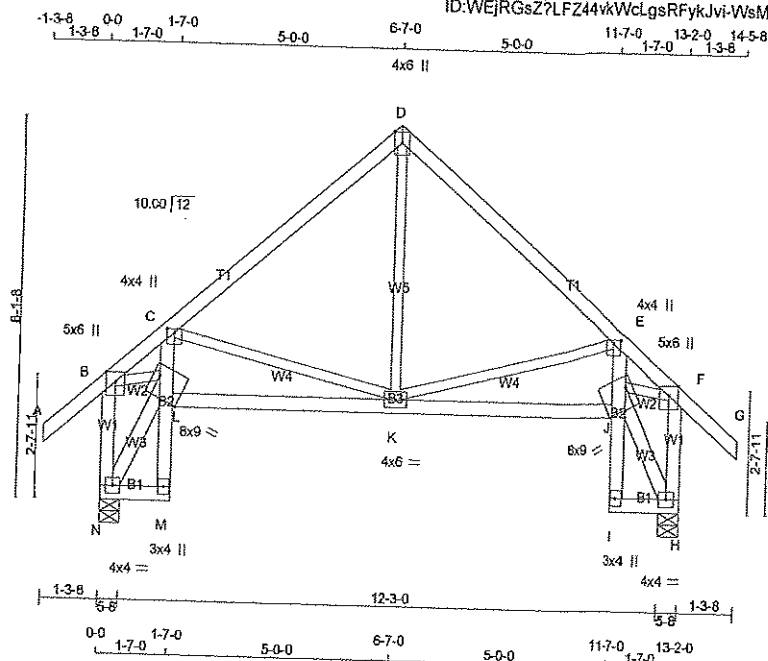
JSI GRIP= 0.75 (D) (INPUT = 0.90)

JSI METAL= 0.30 (D) (INPUT = 1.00)



DRWG NO. TAM 43659.1

STRUCTURAL COMPONENT ONLY



Scale = 1:49.7

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR
A - D	2x4	DRY No.2	SPF
D - G	2x4	DRY No.2	SPF
N - B	2x4	DRY No.2	SPF
H - F	2x4	DRY No.2	SPF
N - M	2x4	DRY No.2	SPF
M - C	2x4	DRY No.2	SPF
L - J	2x4	DRY No.2	SPF
I - E	2x4	DRY No.2	SPF
I - H	2x4	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF
N - L	2x4	DRY No.2	SPF
J - H	2x4	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMWV+p	MT20	5.0	6.0	Edge
C	TMWV+p	MT20	4.0	4.0	1.00 2.00
D	TTW+p	MT20	4.0	6.0	Edge
E	TMWV+p	MT20	4.0	4.0	1.00 2.00
F	TMWV+p	MT20	5.0	6.0	Edge
H	BMWV-1	MT20	4.0	4.0	
I	BMV+p	MT20	3.0	4.0	
J	BVMWV-w	MT20	8.0	9.0	5.25 7.25
K	BVMWV-1	MT20	4.0	6.0	
L	BVMWV-w	MT20	8.0	9.0	5.25 7.25
M	BMV+p	MT20	3.0	4.0	
N	BVMWV-1	MT20	4.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

DESIGNER							
BEARINGS							
FACTORED			MAXIMUM FACTORED			INPUT	REQ'D
GROSS REACTION			GROSS REACTION			BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
N	856	0	856	0	0	5-8	5-8
H	856	0	856	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM/LIVE	WIND	DEAD	SOIL
N	686	408 / 0	138 / 0	0 / 0	0 / 0	140 / 0	0 / 0
H	686	408 / 0	138 / 0	0 / 0	0 / 0	140 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N. H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.55 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MAX. FACTORED		FACTORED		MAX. FACTORED		MAX. FACTORED		
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM TO			FR-TO			
A-B	0/37	-84.3	-84.3	0.12 (1)	10.00	K-D	0/401	0.09 (2)
B-C	-1238/0	-84.3	-84.3	0.20 (1)	5.55	K-E	-625/0	0.30 (1)
C-D	-571/0	-84.3	-84.3	0.28 (1)	6.25	C-K	-625/0	0.30 (1)
D-E	-571/0	-84.3	-84.3	0.28 (1)	6.25	N-L	-31/0	0.60 (1)
E-F	-1238/0	-84.3	-84.3	0.20 (1)	5.55	B-L	0/991	0.22 (1)
F-G	0/37	-84.3	-84.3	0.12 (1)	10.00	J-H	-31/0	0.60 (1)
N-B	-809/0	0.0	0.0	0.11 (1)	7.81	J-F	0/991	0.22 (1)
H-F	-809/0	0.0	0.0	0.11 (1)	7.81			
N-M	0/16	-28.0	-28.0	0.02 (2)	10.00			
M-L	0/35	0.0	0.0	0.06 (1)	10.00			
L-C	0/265	0.0	0.0	0.10 (1)	10.00			
L-K	0/1025	-28.0	-28.0	0.33 (2)	10.00			
K-J	0/1025	-28.0	-28.0	0.33 (2)	10.00			
I-J	0/35	0.0	0.0	0.06 (1)	10.00			
J-E	0/265	0.0	0.0	0.10 (1)	10.00			
I-H	0/16	-28.0	-28.0	0.02 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	OL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	OL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= $L/360$ (0.44")
 CALCULATED VERT. DEFL.(LL) = $L/999$ (0.04")
 ALLOWABLE DEFL.(TL)= $L/360$ (0.44")
 CALCULATED VERT. DEFL.(TL) = $L/999$ (0.08")

CSI: TC=0.28 (C-D:1), BC=0.33 (K-L:2), WB=0.30 (C-K:1), SSI=0.15 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

	GRIP(DRY)		SHEAR		SECTION	
	(PSI)	(PLI)	(PSI)	(PLI)	(PSI)	(PLI)
MT20	618	354	1687	822	2204	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. ≈ 5.0 Deg

SI GRIP= 0.87 (L) (INPUT = 0.50)
SI METAL= 0.23 (F) (INPUT = 1.00)



DWG NO. TAM 4366-17
STRUCTURAL
COMPONENT ONLY

JOB NAME 282698	TRUSS NAME T30	QUANTITY 1	PLY 2	JOB DESC. TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington					
Version 8.030 S Oct 5 2018 MiTek Industries, Inc. Tue Aug 29 11:32:15 2017 Page 2 ID:WmPBIC3LdTRFCMK2RW6ybPykG51-rGBRyKIO2e9IASrdW??SCRZnhr6a7OB83EY?8Tyj0M					

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 278.5 lbs FACTORED DOWN AT 4-5-12, 107.8 lbs FACTORED DOWN AT 4-5-12, 104.1 lbs FACTORED DOWN AT 5-9-4, 104.1 lbs FACTORED DOWN AT 12-10-12, 104.1 lbs FACTORED DOWN AT 14-10-12, AND 104.1 lbs FACTORED DOWN AT 16-3-4, AND 104.1 lbs FACTORED DOWN AT 18-3-4 ON TOP CHORD, AND 86.2 lbs FACTORED DOWN AT 5-11, 73.3 lbs FACTORED DOWN AT 2-5-11, 73.3 lbs FACTORED DOWN AT 4-5-11, 73.3 lbs FACTORED DOWN AT 5-9-4, 644.3 lbs FACTORED DOWN AT 6-1-8, 644.3 lbs FACTORED DOWN AT 12-6-8, 73.3 lbs FACTORED DOWN AT 12-10-12, 73.3 lbs FACTORED DOWN AT 14-10-12, 73.3 lbs FACTORED DOWN AT 16-3-4, AND 73.3 lbs FACTORED DOWN AT 18-3-4, AND 1239.2 lbs FACTORED DOWN AT 18-7-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
F	18-3-4	-104	-104	---	BACK	VERT	TOTAL
R	4-5-11	-42	-73	---	BACK	VERT	TOTAL
T	5-9-4	-104	-104	---	BACK	VERT	TOTAL
U	12-10-12	-104	-104	---	BACK	VERT	TOTAL
V	14-10-12	-104	-104	---	BACK	VERT	TOTAL
W	16-3-4	-104	-104	---	BACK	VERT	TOTAL
X	5-11	-49	-88	---	BACK	VERT	TOTAL
Y	2-5-11	-42	-73	---	BACK	VERT	TOTAL
Z	5-9-4	-42	-73	---	BACK	VERT	TOTAL
AA	6-1-8	-844	-844	---	BACK	VERT	TOTAL
AB	12-6-8	-644	-644	---	BACK	VERT	TOTAL
AC	12-10-12	-42	-73	---	BACK	VERT	TOTAL
AD	14-10-12	-42	-73	---	BACK	VERT	TOTAL
AE	16-3-4	-42	-73	---	BACK	VERT	TOTAL
AF	18-3-4	-42	-73	---	BACK	VERT	TOTAL
AG	18-7-8	-1239	-1239	---	BACK	VERT	TOTAL



8240
P621
DWG NO. TAM 43663-17
STRUCTURAL
COMPONENT ONLY



PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMWV-p	MT20	5.0	8.0	Edge
C	TTWV+m	MT20	5.0	8.0	2.25 1.50
D	TMW+w	MT20	2.0	4.0	
E	TS-t	MT20	3.0	6.0	
F	TMWV-t	MT20	4.0	4.0	
G	TTWV+m	MT20	5.0	8.0	2.25 1.50
H	TMWV-p	MT20	5.0	8.0	Edge
I	BMV1+p	MT20	3.0	6.0	
J	8BMWV-t	MT20	4.0	8.0	2.00 2.75
K	8BMWV-t	MT20	4.0	4.0	2.00 1.50
L	8BMWV-t	MT20	3.0	6.0	
M	BS-t	MT20	4.0	9.0	
N	8BMWVW-t	MT20	4.0	9.0	
O	8BMWV-t	MT20	4.0	6.0	2.00 2.75
P	BMV1+p	MT20	3.0	6.0	

BEARINGS

UNFACTORED REACTIONS

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.86 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL	=	25.6	PSF
		DL	=	3.0	PSF
BOT	CH.	LL	=	10.5	PSF
		DL	=	7.0	PSF
TOTAL LOAD				=	46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= $L/360$ (1.06")
 CALCULATED VERT. DEFL.(LL) = $L/999$ (0.13")
 ALLOWABLE DEFL.(TL)= $L/360$ (1.06")
 CALCULATED VERT. DEFL.(TL) = $L/999$ (0.21")

CSI: TC=0.78 (B-C:1), BC=0.48 (L-N:2), WB=0.45 (F-L:1), SSI=0.26 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

MAIL VALUES

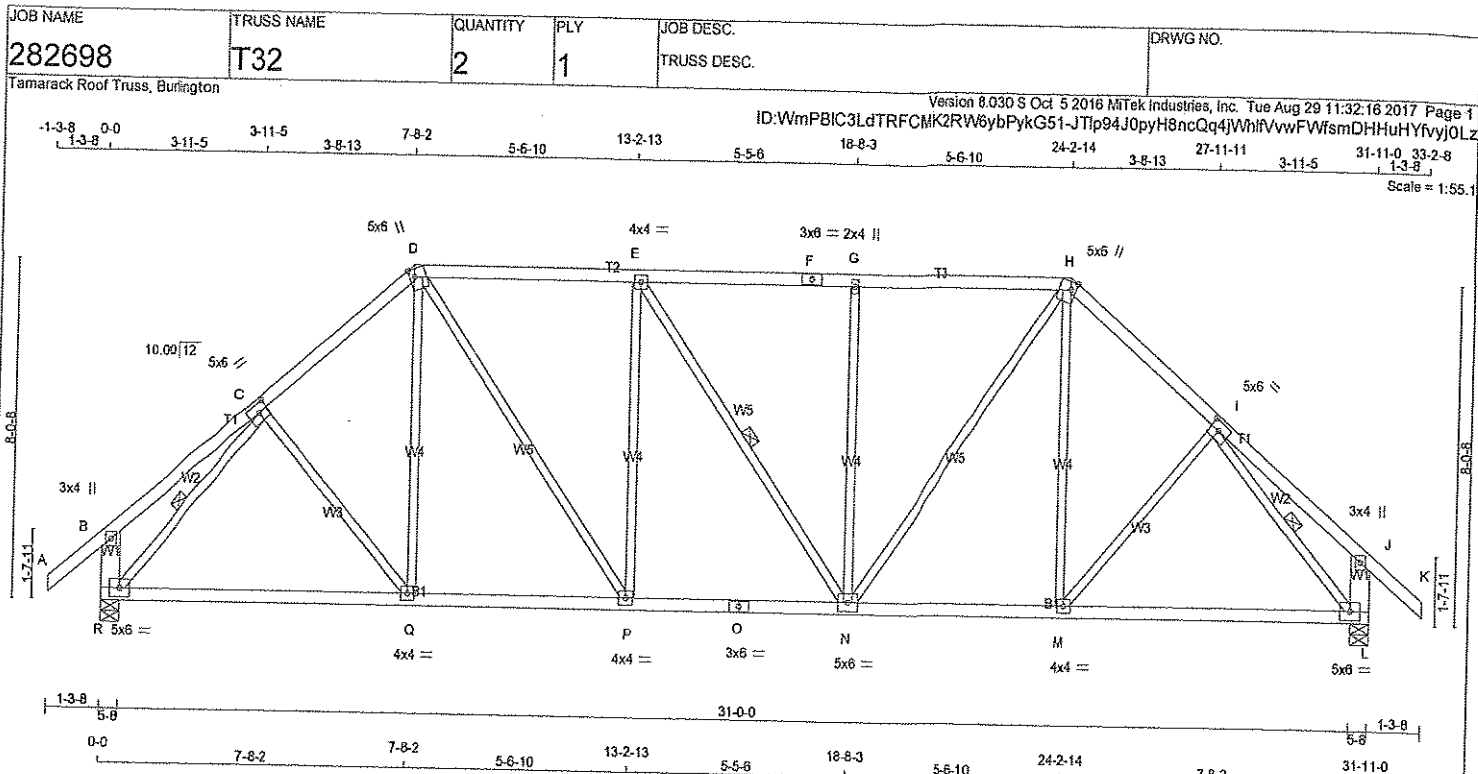
PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

LATE PLACEMENT TOL. = 0.250 inches

LATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.88 (O) (INPUT = 0.90)
SI METAL= 0.69 (M) (INPUT = 1.00)

DWONG, TAM 43664-17
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
R - B	2x6	DRY	No.2
L - J	2x6	DRY	No.2
R - O	2x4	DRY	No.2
O - L	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2

EXCEPT

SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW+1	MT20	5.0	6.0	2.50	2.75
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMVW+1	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMV+w	MT20	2.0	4.0		
H	TTWW+m	MT20	5.0	6.0	2.25	1.50
I	TMVW+1	MT20	5.0	6.0	2.50	2.75
J	TMV+p	MT20	3.0	4.0		
L	BMVW+1	MT20	5.0	6.0		
M, P, Q						
M	BMVW+1	MT20	4.0	4.0		
N	BMVW+1	MT20	5.0	6.0		
O	BS-t	MT20	3.0	6.0		
R	BMVW+1	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	UPLIFT
JT	1908	0	1908	0
R	1908	0	1908	0
L	1908	0	1908	0

UNFACTORED REACTIONS

	1ST LCASE	MAX	MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE	PERM. LIVE
R	1550	887 / 0	335 / 0	0 / 0
L	1550	887 / 0	335 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.49 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N, C-R, H-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

	CHORDS	WEBS
	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO		
A-B	0 / 37	
B-C	0 / 23	
C-D	-1811 / 0	
D-E	-1829 / 0	
E-F	-1828 / 0	
F-G	-1828 / 0	
G-H	-1828 / 0	
H-I	-1811 / 0	
I-J	0 / 22	
J-K	0 / 37	
R-B	-242 / 0	
L-J	-242 / 0	
R-Q	0 / 1341	
Q-P	0 / 1373	
P-O	0 / 1829	
O-N	0 / 1829	
N-M	0 / 1373	
M-L	0 / 1341	

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 3.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.06")

CALCULATED VERT. DEFL. (LL) = L/999 (0.16")

ALLOWABLE DEFL. (TL) = L/360 (1.06")

CALCULATED VERT. DEFL. (TL) = L/999 (0.26")

CSI: TC=0.43 (D-E:1), BC=0.53 (M-N:2), WB=0.62 (E-P:1), SSI=0.22 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

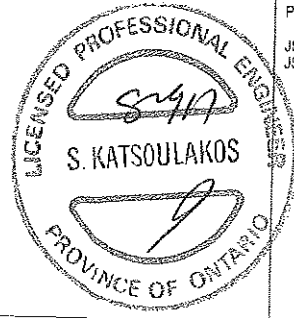
MT20 618 354 1657 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.88 (I) (INPUT = 0.90)

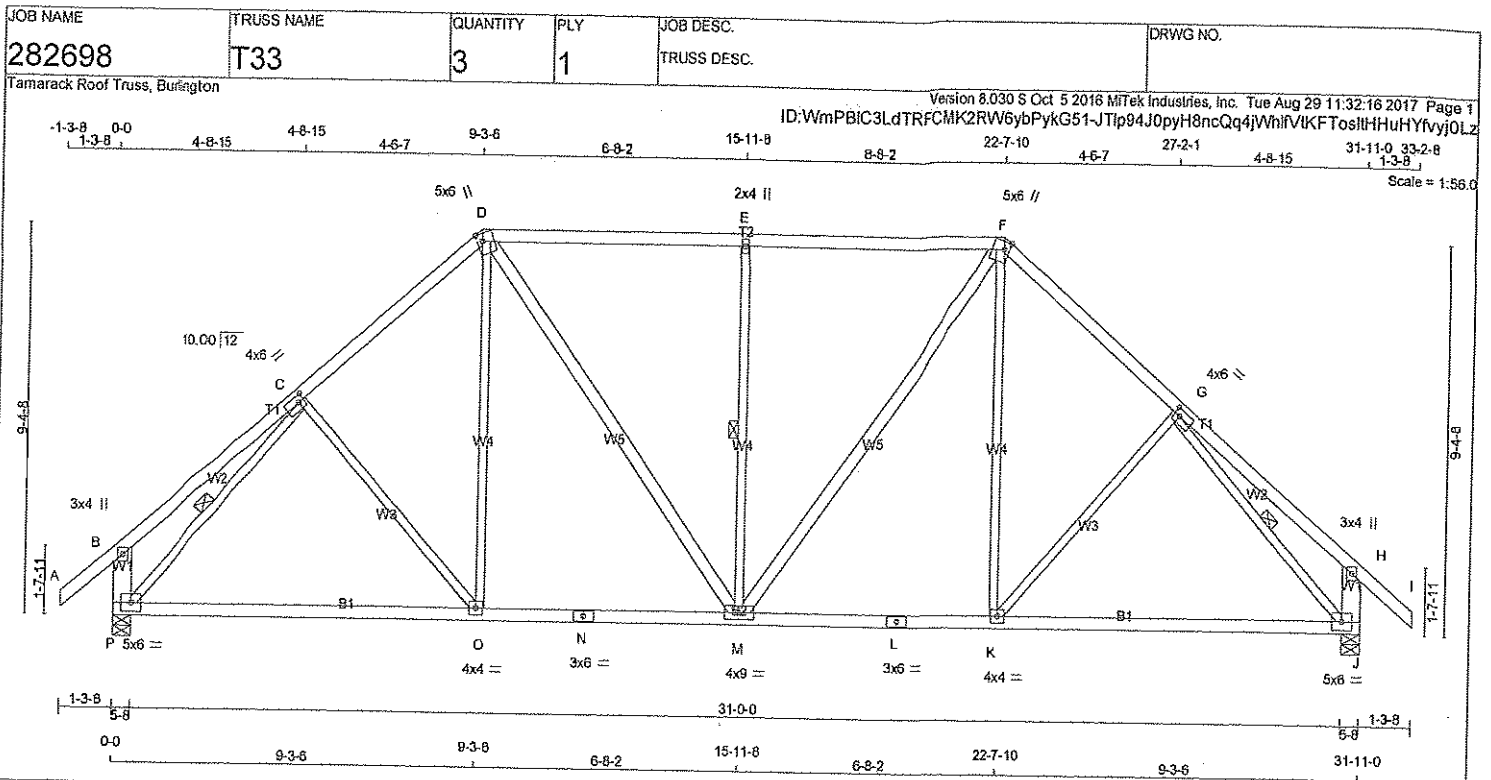
JSI METAL= 0.59 (O) (INPUT = 1.00)



DWG NO. TAM 43665-17

STRUCTURAL

COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - I	2x4	DRY	No.2
P - B	2x6	DRY	No.2
J - H	2x6	DRY	No.2
P - N	2x4	DRY	No.2
N - L	2x4	DRY	No.2
L - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
D - M	2x4	DRY	No.2
M - F	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	8.0	2.00	1.75
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	4.0	6.0	2.00	1.75
H	TMV+p	MT20	3.0	4.0		
J	BMWW-t	MT20	5.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	9.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	JT VERT	UPLIFT	IN-SX
P 1908 0	1908 0	5-8	5-8
J 1908 0	1908 0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX	MIN	COMPONENT REACTIONS
JT COMBINED	SNOW	LIVE	PERM.LIVE
P	1550	887 / 0	335 / 0
J	1550	887 / 0	335 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.54 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-M, C-P, G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	CSF (LC)
FR-TO				FR-TO			
A-B	0 / 137	-84.3	-84.3 0.12 (1)	C-O	-75 / 104	0.07 (1)	
B-C	0 / 129	-84.3	-84.3 0.29 (1)	D-D	0 / 443	0.10 (2)	
C-D	-1756 / 0	-84.3	-84.3 0.27 (1)	D-M	0 / 505	0.08 (1)	
D-E	-1628 / 0	-84.3	-84.3 0.53 (1)	M-E	-589 / 0	0.40 (1)	
E-F	-1628 / 0	-84.3	-84.3 0.53 (1)	M-F	0 / 505	0.08 (1)	
F-G	-1756 / 0	-84.3	-84.3 0.27 (1)	K-F	0 / 443	0.10 (2)	
G-H	0 / 29	-84.3	-84.3 0.29 (1)	K-G	-75 / 104	0.07 (1)	
H-I	0 / 137	-84.3	-84.3 0.12 (1)	P-C	-2081 / 0	0.64 (1)	
P-B	-266 / 0	0.0	0.0 0.02 (1)	G-J	-2081 / 0	0.64 (1)	
J-H	-266 / 0	0.0	0.0 0.02 (1)				
P-O	0 / 1375	-28.0	-28.0 0.71 (2)				
O-N	0 / 1328	-28.0	-28.0 0.71 (2)				
N-M	0 / 1328	-28.0	-28.0 0.71 (2)				
M-L	0 / 1328	-28.0	-28.0 0.71 (2)				
L-K	0 / 1328	-28.0	-28.0 0.71 (2)				
K-J	0 / 1375	-28.0	-28.0 0.71 (2)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.8 PSF

DL = 3.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.06")

CALCULATED VERT. DEFL.(LL) = L/999 (0.30")

ALLOWABLE DEFL.(TL) = L/360 (1.06")

CALCULATED VERT. DEFL.(TL) = L/767 (0.50")

CSI: TC=0.53 (D-E:1), BC=0.71 (M-O:2), WB=0.64 (C-P:1), SSI=0.27 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

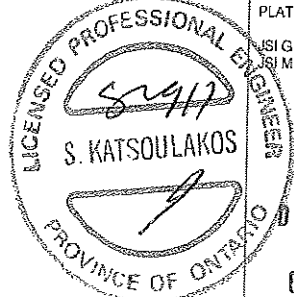
MT20 618 354 1867 822 2284 1856

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

SSI GRIP= 0.90 (C) (INPUT = 0.90)

SSI METAL= 0.51 (G) (INPUT = 1.00)



DRWG NO. TAM 43666 17

STRUCTURAL COMPONENT ONLY

282698

TRUSS NAME
T34

QUANTITY
1

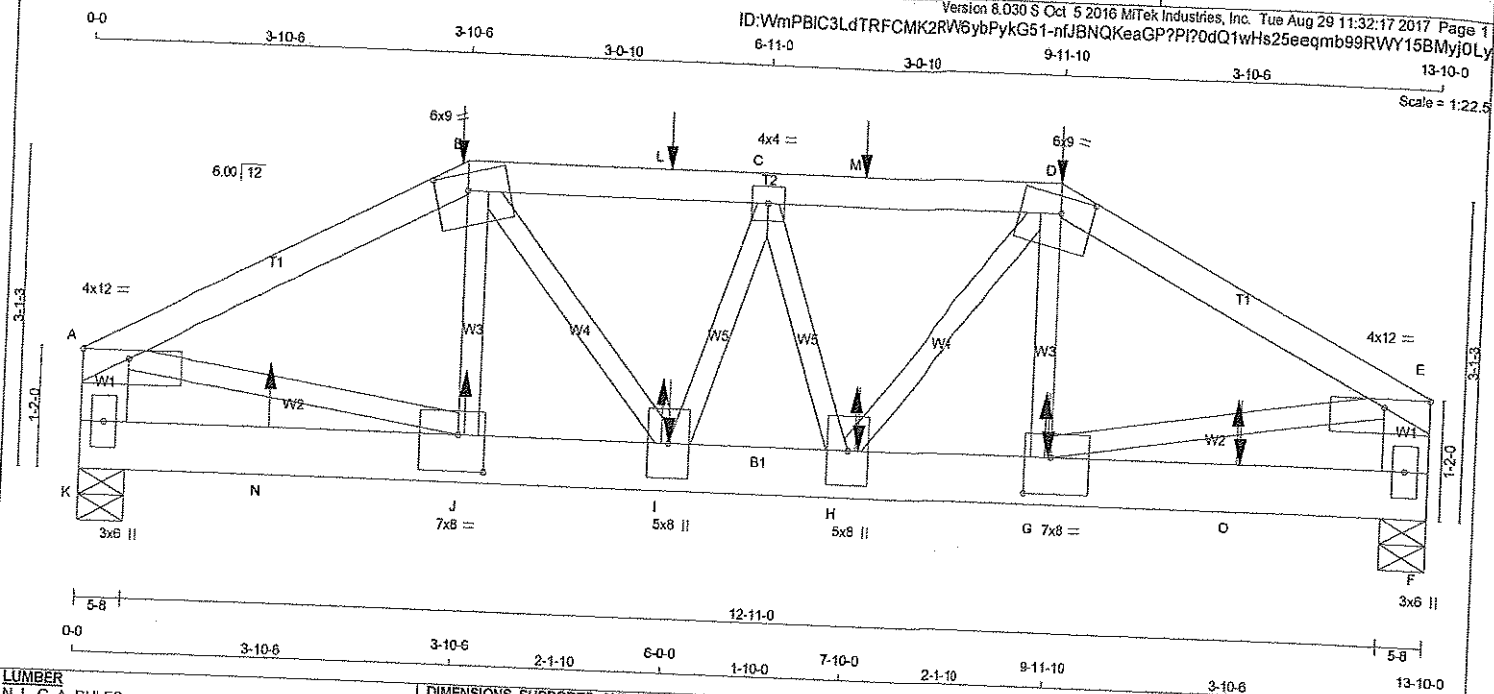
PLY
2

JOB DESC.
TRUSS DESC.

DRWG NO.

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Aug 29 11:32:17 2017 Page 1
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JOB NAME 282698	TRUSS NAME T34	QUANTITY 1	PLY 2	JOB DESC. TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

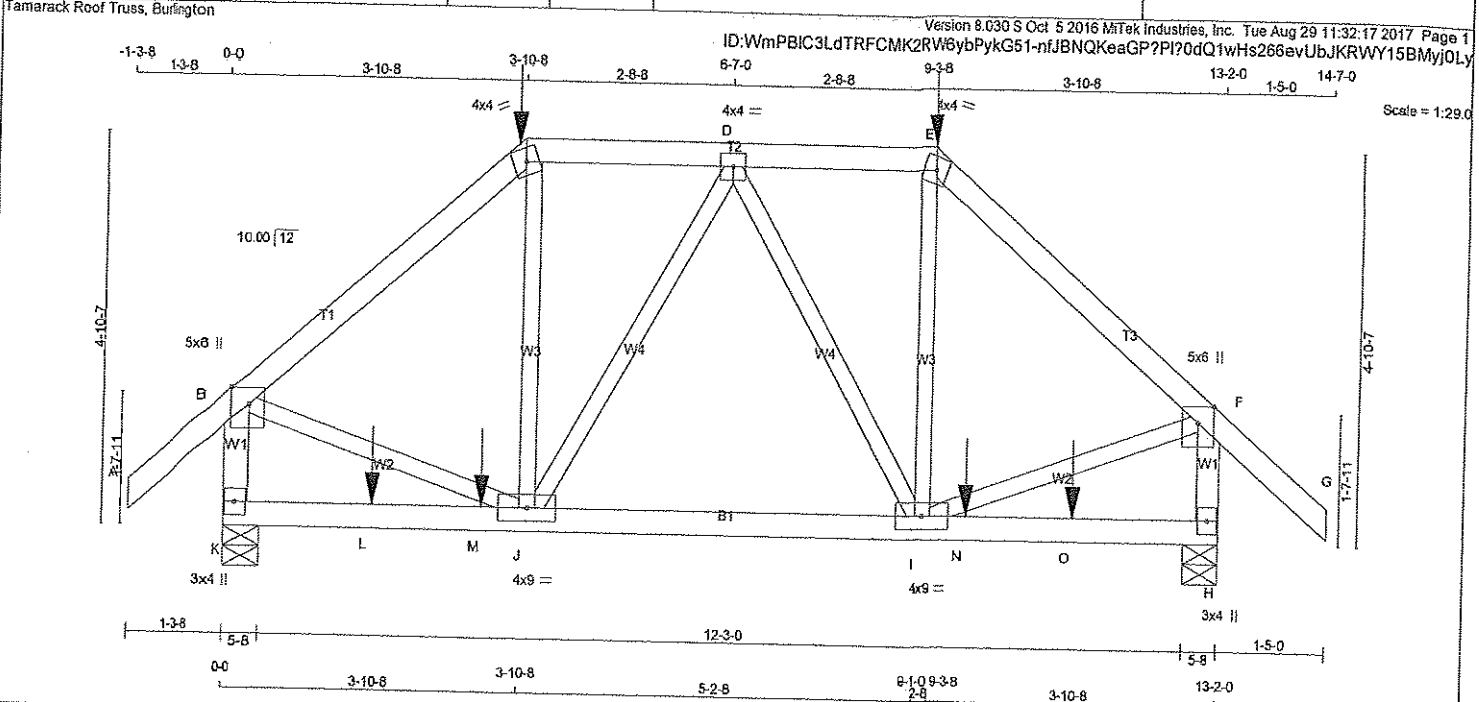
Version 8.030 S Oct 5 2016 Mitek Industries, Inc. Tue Aug 29 11:32:17 2017 Page 2
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HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1.6 lbs FACTORED DOWN AND 108.5 lbs FACTORED UP AT 3-10-6, 1.5 lbs FACTORED DOWN AND 108.6 lbs FACTORED UP AT 5-11-2, AND 1.5 lbs FACTORED DOWN AND 108.6 lbs FACTORED UP AT 7-10-14, AND 1.6 lbs FACTORED DOWN AND 108.5 lbs FACTORED UP AT 9-11-10 ON TOP CHORD, AND 7.5 lbs FACTORED UP AT 1-11-2, 7.5 lbs FACTORED UP AT 3-11-2, 7.5 lbs FACTORED UP AT 5-11-2, 2807.8 lbs FACTORED DOWN AT 6-0-0, 7.5 lbs FACTORED UP AT 7-10-14, 1877.7 lbs FACTORED DOWN AT 7-11-4, 7.5 lbs FACTORED UP AT 9-10-14, 1877.7 lbs FACTORED DOWN AT 9-11-4, AND 7.5 lbs FACTORED UP AT 11-10-14, AND 1877.7 lbs FACTORED DOWN AT 11-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.



DWG NO. TAM 43662-17
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - E	2x4	DRY No.2	SPF
E - G	2x4	DRY No.2	SPF
K - B	2x4	DRY No.2	SPF
H - F	2x4	DRY No.2	SPF
K - H	2x4	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTW-m	MT20	4.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TTW-m	MT20	4.0	4.0		
F	TMVW+p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	9.0		
J	BMWW-t	MT20	4.0	9.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 183.4 lbs FACTORED DOWN AT 3-10-8 AND 183.4 lbs FACTORED DOWN AT 9-3-8 ON TOP CHORD, AND 42.0 lbs FACTORED DOWN AT 1-11-4, 409.2 lbs FACTORED DOWN AT 3-4-8, AND 409.2 lbs FACTORED DOWN AT 9-9-8, AND 42.0 lbs FACTORED DOWN AT 11-2-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ UPLIFT		
K	1472	0	1472	0	5-8	5-8
H	1483	0	1483	0	5-8	5-8

UNFACTORED REACTIONS						
	1ST LCASE	MAX/MIN COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
K	1164	718 / 0	217 / 0	0 / 0	0 / 0	229 / 0
H	1172	725 / 0	217 / 0	0 / 0	0 / 0	230 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.44 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO		
A-B	0 / 37	-84.3 -84.3 0.13 (1)	10.00	J-C 0 / 353 0.09 (2)
B-C	-1223 / 0	-84.3 -84.3 0.28 (1)	5.44	I-E 0 / 353 0.09 (2)
C-D	-944 / 0	-84.3 -84.3 0.11 (1)	6.24	B-J 0 / 586 0.24 (1)
D-E	-944 / 0	-84.3 -84.3 0.11 (1)	6.24	I-F 0 / 586 0.24 (1)
E-F	-1223 / 0	-84.3 -84.3 0.28 (1)	5.44	J-D -149 / 0 0.07 (1)
F-G	0 / 41	-84.3 -84.3 0.15 (1)	10.00	D-I -149 / 0 0.07 (1)
K-B	-1373 / 0	0.0 0.0 0.16 (1)	6.90	
H-F	-1384 / 0	0.0 0.0 0.15 (1)	6.88	
K-L	0 / 0	-28.0 -28.0 0.23 (2)	10.00	
L-M	0 / 0	-28.0 -28.0 0.23 (2)	10.00	
M-J	0 / 0	-28.0 -28.0 0.23 (2)	10.00	
J-I	0 / 1018	-28.0 -28.0 0.36 (1)	10.00	
I-N	0 / 0	-28.0 -28.0 0.23 (2)	10.00	
N-O	0 / 0	-28.0 -28.0 0.23 (2)	10.00	
O-H	0 / 0	-28.0 -28.0 0.23 (2)	10.00	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE
C	3-10-8	-183	-183	---	FRONT	VERT	TOTAL
E	9-3-8	-183	-183	---	FRONT	VERT	TOTAL
L	1-11-4	-24	-24	---	FRONT	VERT	TOTAL
M	3-4-8	-409	-409	---	FRONT	VERT	TOTAL
N	9-9-8	-409	-409	---	FRONT	VERT	TOTAL
O	11-2-12	-24	-24	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	PSF
DL	25.6	PSF
BOT CH.	LL	PSF
DL	10.5	PSF
TL	7.0	PSF
TOTAL LOAD	46.1	PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.44")

CALCULATED VERT. DEFL.(LL)= L/999 (0.04")

ALLOWABLE DEFL.(TL)= L/360 (0.44")

CALCULATED VERT. DEFL.(TL)= L/999 (0.07")

CSI: TC=0.28 (B-C:1), BC=0.38 (I-J:1), WB=0.24 (B-J:1), SSI=0.38 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00

COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

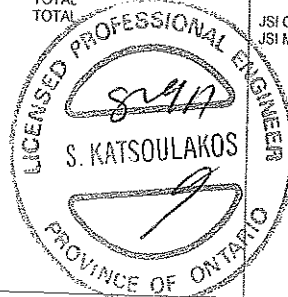
PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	MAX MIN	MAX MIN
MT20	618	354	1687	822	2284 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg

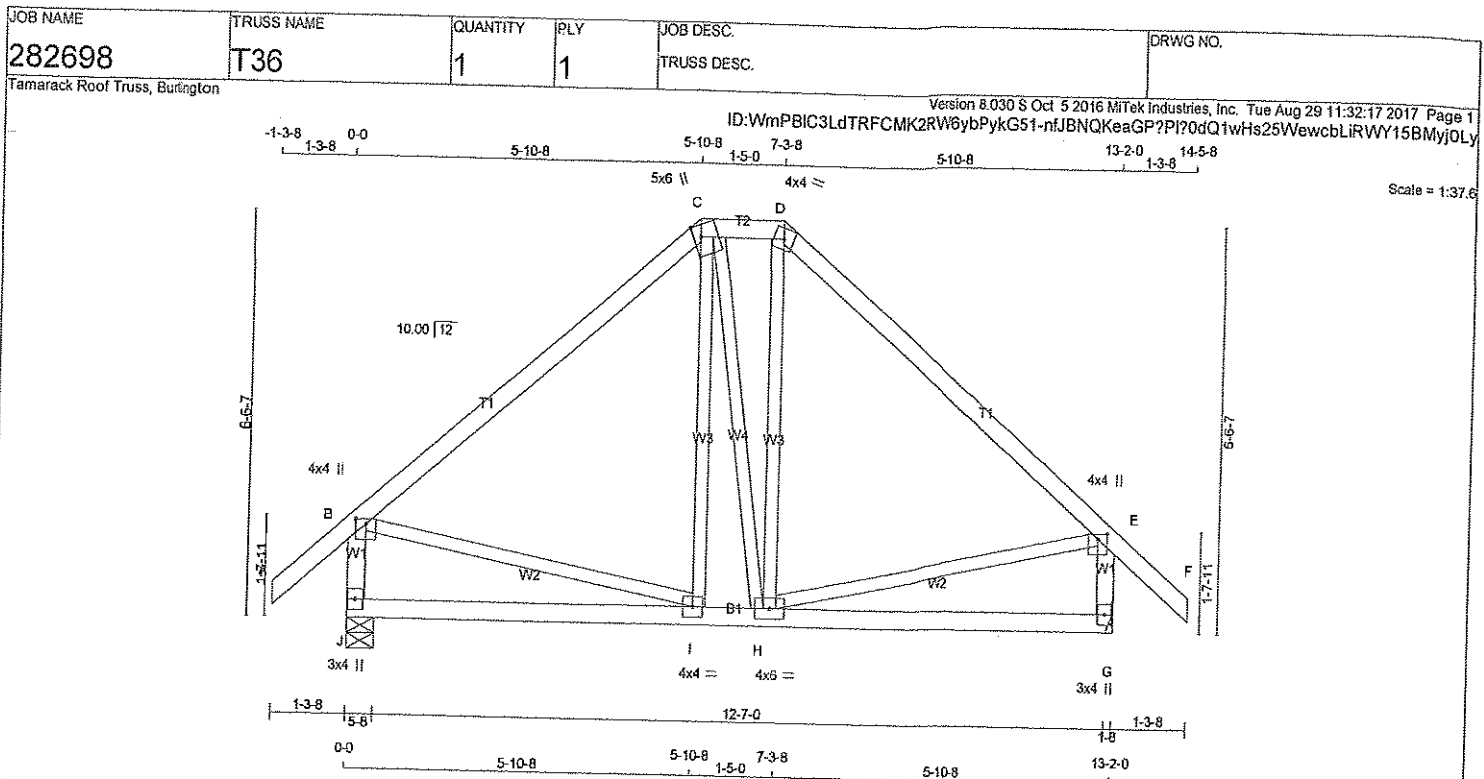
JSI GRIP= 0.64 (F) (INPUT = 0.90)

JSI METAL= 0.24 (F) (INPUT = 1.00)



DWG NO. TAM 4366B 17

STRUCTURAL COMPONENT ONLY



LUMBER

N. L. G. A. RULES
CHORDS SIZE LUMBER
A - C 2x4 DRY No.2
C - D 2x4 DRY No.2
D - F 2x4 DRY No.2
J - B 2x4 DRY No.2
G - E 2x4 DRY No.2
J - G 2x4 DRY No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW+p	MT20	4.0	4.0	1.00	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMW+p	MT20	4.0	4.0	1.00	2.00
G	BMV+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMV+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	IN-SX	IN-SX
J	856	0	5-8	5-8
G	856	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX / MIN	COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
J	856	408 / 0	138 / 0	0 / 0	0 / 0	0 / 0	0 / 0	140 / 0	0 / 0
G	856	408 / 0	138 / 0	0 / 0	0 / 0	0 / 0	0 / 0	140 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRACED LENGTH	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO						FR-TO			
A-B	0 / 37	-84.3	-84.3	0.12 (1)	10.00	I-C	0 / 141	0.03 (3)	
B-C	-523 / 0	-84.3	-84.3	0.38 (1)	6.25	C-H	0 / 16	0.00 (3)	
C-D	-403 / 0	-84.3	-84.3	0.02 (1)	6.25	H-D	0 / 156	0.04 (3)	
D-E	-524 / 0	-84.3	-84.3	0.38 (1)	6.25	B-I	0 / 411	0.09 (1)	
E-F	0 / 37	-84.3	-84.3	0.12 (1)	10.00	H-E	0 / 411	0.09 (1)	
J-B	-788 / 0	0.0	0.0	0.09 (1)	7.81				
G-E	-787 / 0	0.0	0.0	0.08 (1)	7.81				
J-I	0 / 0	-28.0	-28.0	0.23 (3)	10.00				
I-H	0 / 402	-28.0	-28.0	0.29 (2)	10.00				
H-G	0 / 0	-28.0	-28.0	0.25 (3)	10.00				

TOTAL WEIGHT = 65 lb

[MIF]

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.44")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.44")
CALCULATED VERT. DEFL.(TL) = L/999 (0.08")

CSI: TC=0.38 (D-E:1), BC=0.29 (H-I:2), WB=0.09 (E-H:1), SS=0.15 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

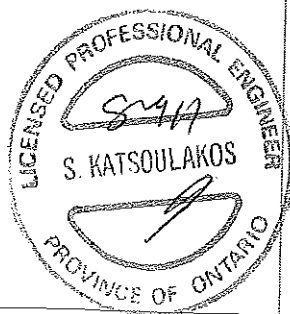
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1657 822 2284 1656

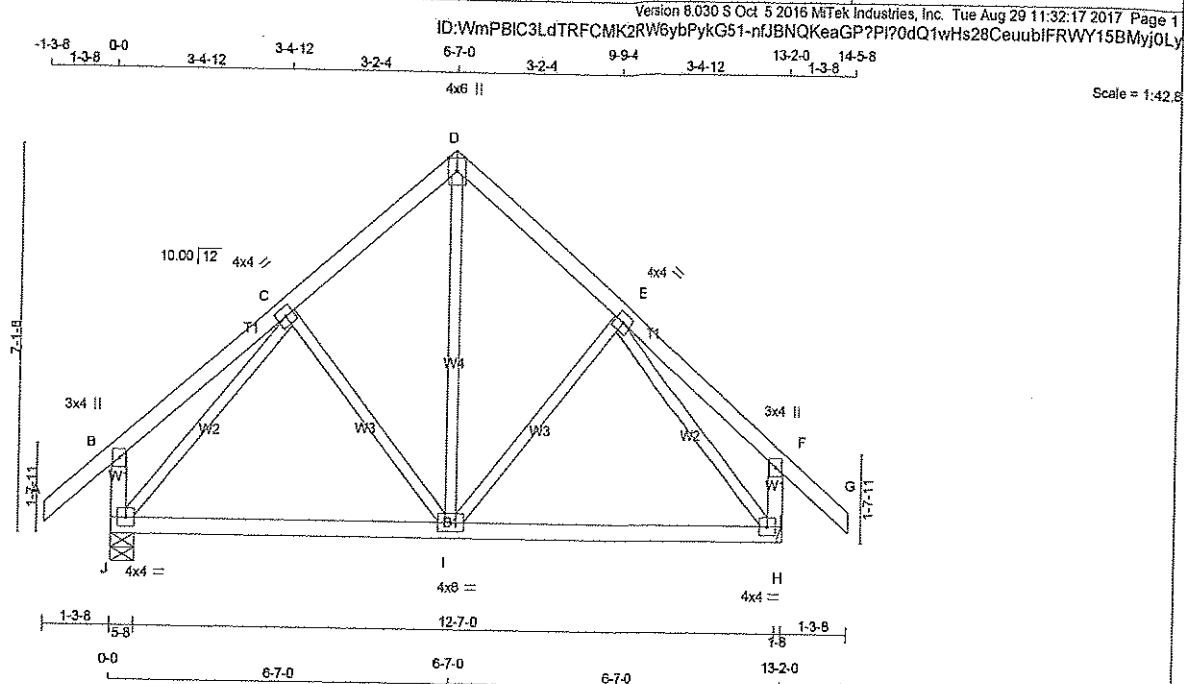
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.73 (E) (INPUT = 0.90)
JSI METAL= 0.20 (E) (INPUT = 1.00)



DRWG NO. TAM 43669-17
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
J - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
J - H	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	4.0	4.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMVW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMVW1-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
J	856	0	856	0	5-8	5-8
H	856	0	856	0	5-8	5-8

HANGER BY OTHERS
MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	686	408 / 0	138 / 0	0 / 0	0 / 0	140 / 0	0 / 0
H	686	408 / 0	138 / 0	0 / 0	0 / 0	140 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM	TO		FR-TO			
A-B	0 / 37	-84.3	-84.3	0.12 (1)	10.00	I-D	0 / 414	0.09 (1)
B-C	0 / 20	-84.3	-84.3	0.14 (1)	10.00	I-E	-121 / 39	0.05 (1)
C-D	-514 / 0	-84.3	-84.3	0.11 (1)	6.25	C-I	-121 / 39	0.05 (1)
D-E	-514 / 0	-84.3	-84.3	0.11 (1)	6.25	J-C	-724 / 0	0.31 (1)
E-F	0 / 20	-84.3	-84.3	0.14 (1)	10.00	E-H	-724 / 0	0.31 (1)
F-G	0 / 37	-84.3	-84.3	0.12 (1)	10.00			
J-B	-223 / 0	0.0	0.0	0.02 (1)	7.81			
H-F	-223 / 0	0.0	0.0	0.02 (1)	7.81			
J-I	0 / 456	-28.0	-28.0	0.40 (2)	10.00			
I-H	0 / 456	-28.0	-28.0	0.40 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 3.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2012, CBC 2012, ABC 2014
- CSA C88-09
- TPC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.44")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.06")
ALLOWABLE DEFL.(TL) = $L/360$ (0.44")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.09")

CSI: TC=0.14 (E-F:1), BC=0.40 (H-I:2), WB=0.31 (E-H:1), SSI=0.16 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

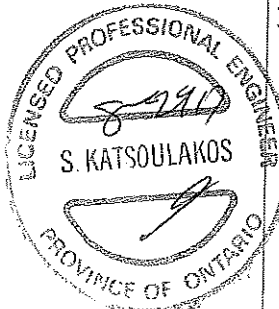
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1687 822 2284 1655

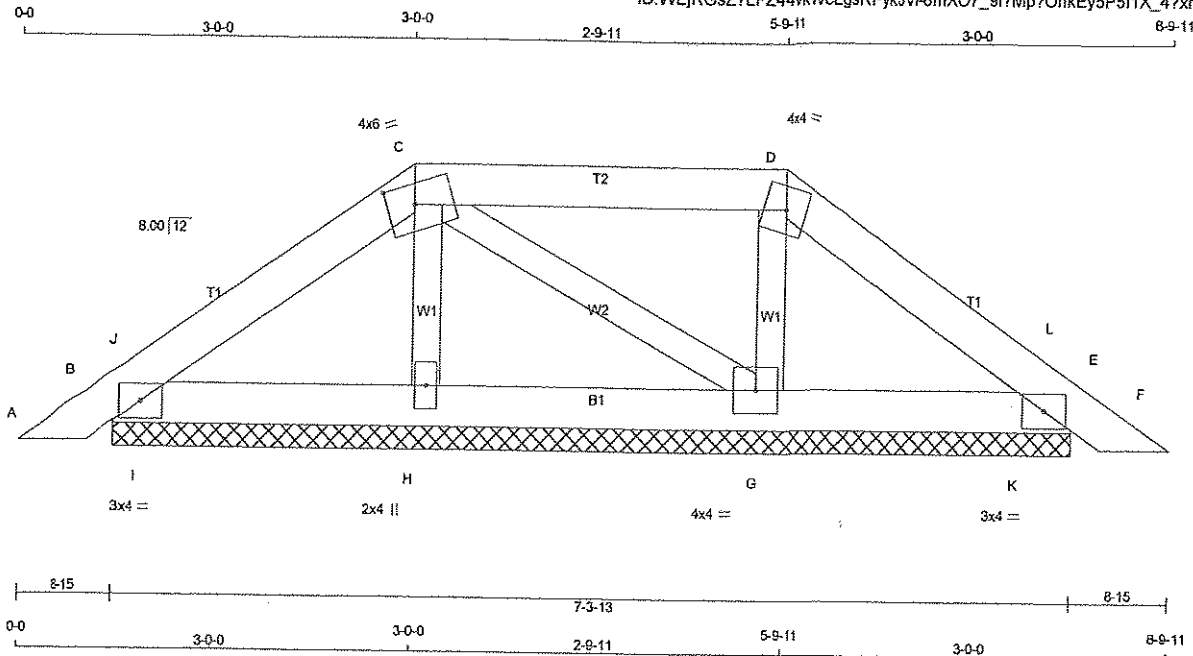
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (C) (INPUT = 0.90)
JSI METAL= 0.27 (C) (INPUT = 1.00)



DRWG NO. TAM43670-17
STRUCTURAL
COMPONENT ONLY



LUMBER
N. L. G. A. RULES
CHORDS SIZE
A - C 2x4 DRY
C - D 2x4 DRY
D - F 2x4 DRY
B - E 2x4 DRY
ALL WEBS 2x3 DRY
DRY: SEASONED LUMBER

LUMBER DESCR.
No.2 SPF
No.2 SPF
No.2 SPF
No.2 SPF
No.2 SPF

PLATES (table is in inches)
JT TYPE PLATES W LEN Y X
B TMB1-I MT20 3.0 4.0
C TTWW-m MT20 4.0 6.0 1.75 2.50
D TTW-m MT20 4.0 4.0
E TMB1-I MT20 3.0 4.0
G BMWW1-I MT20 4.0 4.0
H BMW1-w MT20 2.0 4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	GROSS REACTION	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
B	214	0	214	0	7-3-13
E	202	0	202	0	7-3-13
H	242	0	242	0	7-3-13
G	275	0	275	0	7-3-13

UNFACTORED REACTIONS

JT	COMBINED	1ST LCASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	181	113 / 0	21 / 0	0 / 0	0 / 0	27 / 0	0 / 0	0 / 0	0 / 0
E	154	106 / 0	21 / 0	0 / 0	0 / 0	27 / 0	0 / 0	0 / 0	0 / 0
H	207	102 / 0	55 / 0	0 / 0	0 / 0	49 / 0	0 / 0	0 / 0	0 / 0
G	229	122 / 0	55 / 0	0 / 0	0 / 0	51 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)
FR-TO									
A-B	0 / 14	-64.3	-84.3	0.03 (1)	10.00	H-C	-151 / 0	0.02 (1)	
B-J	-64 / 0	-84.3	-84.3	0.01 (3)	6.25	C-G	-21 / 0	0.00 (1)	
J-C	-69 / 0	-84.3	-84.3	0.05 (1)	8.25	G-D	-172 / 0	0.03 (1)	
C-D	-28 / 0	-84.3	-84.3	0.11 (1)	6.25	I-J	-102 / 27	0.00 (1)	
D-L	-49 / 0	-84.3	-84.3	0.05 (1)	6.25	K-L	-104 / 27	0.00 (1)	
L-E	-43 / 0	-84.3	-84.3	0.01 (3)	6.25				
E-F	0 / 14	-84.3	-84.3	0.03 (1)	10.00				
B-I	0 / 56	-28.0	-28.0	0.05 (1)	10.00				
I-H	0 / 56	-28.0	-28.0	0.05 (1)	10.00				
H-G	0 / 46	-28.0	-28.0	0.05 (2)	10.00				
G-K	0 / 40	-28.0	-28.0	0.05 (1)	10.00				
K-E	0 / 40	-28.0	-28.0	0.05 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11 (C-D:1), BC=0.05 (H-I:1), WB=0.03 (D-G:1), SSI=0.09 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 619 354 1657 822 2284 1655

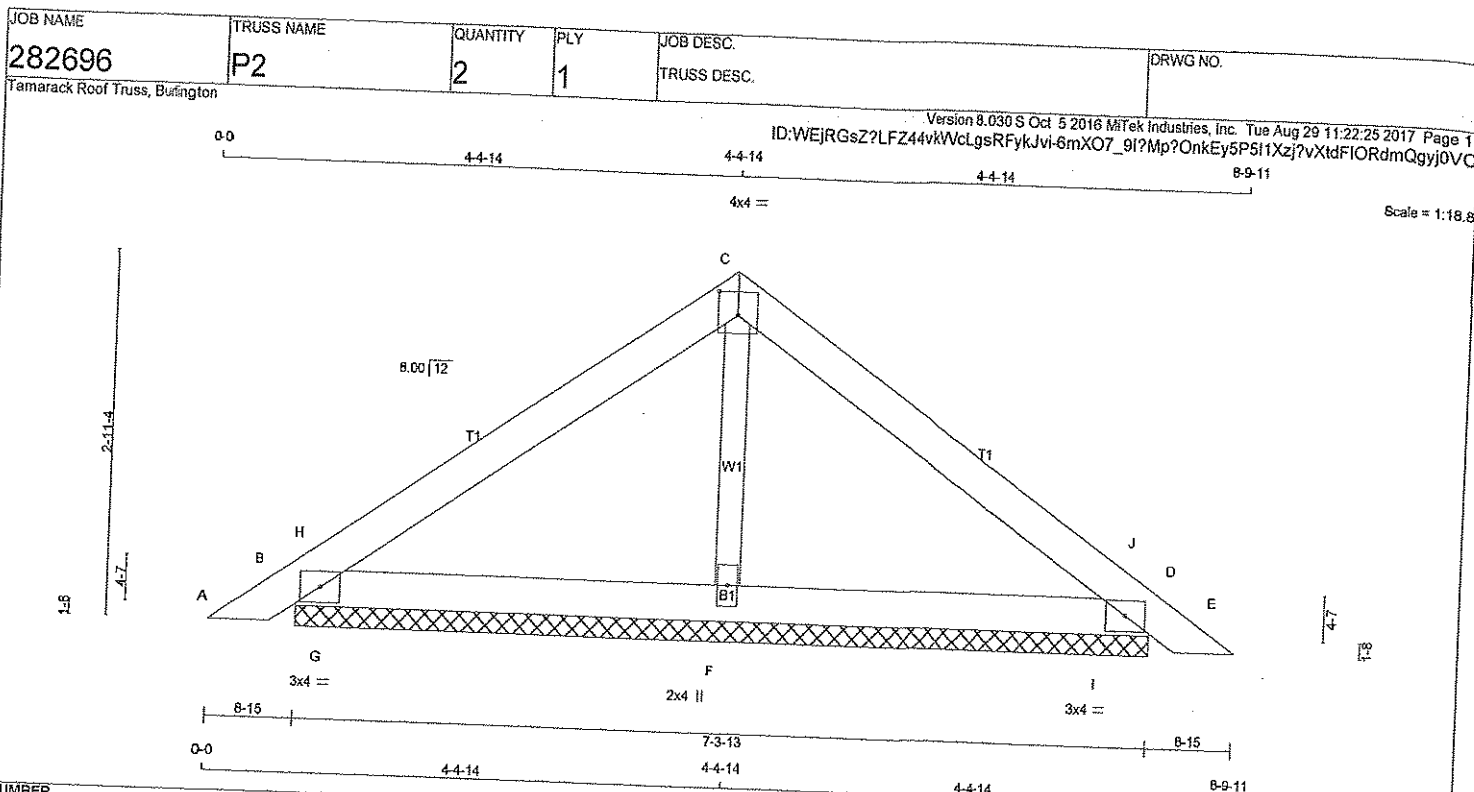
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (B) (INPUT = 0.50)
JSI METAL= 0.04 (B) (INPUT = 1.00)



DWG NO. TAM 4364717
STRUCTURAL
COMPONENT ONLY



LUMBER
N. L. G. A. RULES
CHORDS SIZE
A - C 2x4 DRY
C - E 2x4 DRY
B - D 2x4 DRY
ALL WEBS 2x3 DRY
DRY: SEASONED LUMBER.

LUMBER
No.2
No.2
No.2
No.2

DESCR.
SPF
SPF
SPF
SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
	VERT	HORZ	UPLIFT	IN-SX
B	302	0	0	7-3-13
D	302	0	0	7-3-13
F	330	0	0	7-3-13

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS				WIND	DEAD	SOIL
		COMBINED	SNOW	LIVE	PERM. LIVE			
B	232	154 / 0	36 / 0	0 / 0	0 / 0	0 / 0	42 / 0	0 / 0
D	232	154 / 0	36 / 0	0 / 0	0 / 0	0 / 0	42 / 0	0 / 0
F	287	133 / 0	63 / 0	0 / 0	0 / 0	0 / 0	71 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	1ST LCASE	MAX. FACTORED FORCE (LBS)	MEMB.	WEBS MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO							
A-B	0 / 14	-84.3	-84.3	0.03 (1)	10.00	F-C	-131 / 0
B-H	-70 / 0	-84.3	-84.3	0.05 (1)	6.25	G-H	-283 / 53
H-C	-121 / 0	-84.3	-84.3	0.14 (1)	6.25	I-J	-283 / 53
C-J	-121 / 0	-84.3	-84.3	0.14 (1)	6.25		
J-D	-70 / 0	-84.3	-84.3	0.05 (1)	6.25		
D-E	0 / 14	-84.3	-84.3	0.03 (1)	10.00		
B-G	0 / 95	-28.0	-28.0	0.13 (1)	10.00		
G-F	0 / 95	-28.0	-28.0	0.14 (1)	10.00		
F-I	0 / 95	-28.0	-28.0	0.14 (1)	10.00		
I-D	0 / 95	-28.0	-28.0	0.13 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 085-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14 (C-H:1), BC=0.14 (F-G:1), WB=0.02 (C-F:1), SSI=0.22 (D-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)



DWG NO. TAM 4364817
STRUCTURAL
COMPONENT ONLY

LUS - Double Shear Joist Hangers



All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

MATERIAL: 18 gauge

FINISH: G90 galvanized

DESIGN:

- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

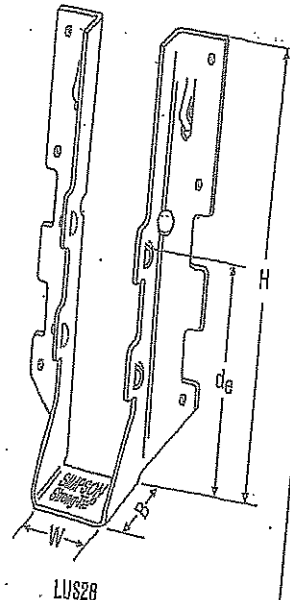
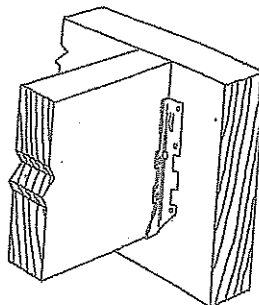
INSTALLATION:

- Use all specified fasteners
- Nails: 16d = 0.162" dia, x 3 1/2" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

- These hangers cannot be modified.

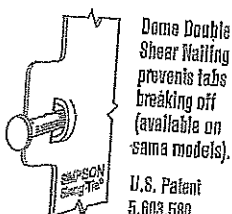
Typical LUS Installation



LUS26

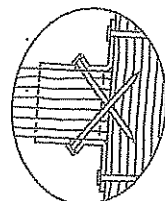
Model No.	Ga	Dimensions (in)					Fasteners		Factored Resistance (lbs)			
		W	H	B	dg ¹	Face	Joist		D.Fir-L		S-P-F	
									Uplift (K _p =1.15)	Normal (K _p =1.00)	Uplift (K _p =1.15)	Normal (K _p =1.00)
LUS24	18	1 1/8	3 1/8	1 1/8	1 1/8	4-10d	2-16d		710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 1/8	4-16d	2-16d		835	2020	590	1435
LUS26	18	1 1/8	4 3/4	1 1/8	3/8	4-10d	4-10d		1420	2170	1290	1630
LUS26-2	18	3 1/8	4 3/4	2	4	4-16d	4-16d		1720	2595	1545	1920
LUS26-3	18	4 3/8	4 3/8	2	3/4	4-16d	4-16d		1720	2595	1545	1920
LUS28	18	1 1/8	6 3/8	1 1/8	3/4	6-10d	4-10d		1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	6-16d	4-16d		1720	3325	1545	2575
LUS28-3	18	4 3/8	6 3/4	2	3/4	6-16d	4-16d		1720	3325	1545	2375
LUS210	18	1 1/8	7 1/8	1 1/8	3/8	8-10d	4-10d		1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	8-16d	6-16d		2580	4500	2320	3195
LUS210-3	18	4 3/8	8 3/8	2	5 1/4	8-16d	6-16d		2580	3345	2320	2375

¹ dg is the distance from the seat of the hanger to the highest joist nail.

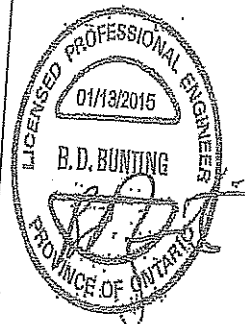


Dome Double Shear Nailing prevents tabs breaking off (available on same models).

U.S. Patent
5,603,580



Double Shear Nailing
Top View.



This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2015. This information is updated periodically and should not be relied upon after December 31, 2016. Contact Simpson Strong-Tie for current information and limited warranty or see www.strongtie.com.

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T-SPECUS16 1/15 rev. 12/16

800-999-5099

www.strongtie.com

HUS/LJS - Double Shear Joist Hangers



All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: See table

FINISH: G90 galvanized

DESIGN:

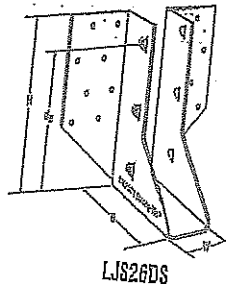
- Factored resistances are in accordance with CSA 086-14
- Uplift resistances have been increased 15%. No further increase is permitted
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

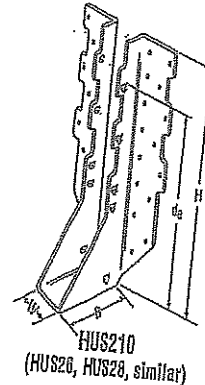
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/4" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

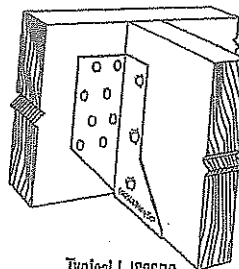
- See current catalogue for options



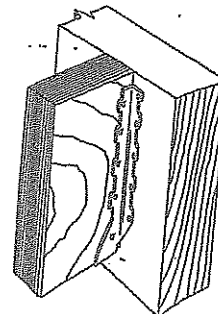
LJS26DS



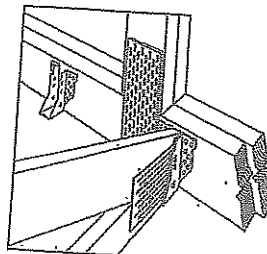
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



Typical HUS
Installation

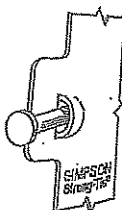


Typical HUS Installation
(Truss Designer to provide fastener
quantity for connecting multiple
members together)

Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
		W	H	B	d _g ¹	Face	Joist	D-Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
								(K _g =1.15)	(K _g =1.00)	(K _g =1.15)	(K _g =1.00)
LJS26DS	18	1 7/16	6	3 1/2	4 3/4	16-16d	6-16d	2055	4265	1460	4115
HUS26	18	1 7/8	5 3/4	3	3 1/8	14-16d	6-16d	2705	4940	2065	3875
HUS28	16	1 5/8	7 3/4	3	6 3/4	22-16d	8-16d	3605	5365	2875	4345
HUS210	18	1 7/8	9 3/4	3	7 3/4	30-16d	10-16d	4505	5795	4010	4740
HUS1.81/10	16	1 1/4	9	3	8	30-16d	10-16d	4505	6450	4010	5200

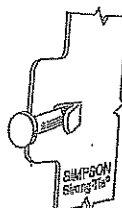
1. d_g is the distance from the seat of the hanger to the highest fastener.

1. d_g is the distance from the seat of the hanger to the highest joist nail.

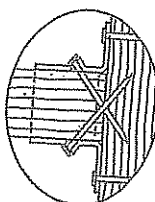


Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).

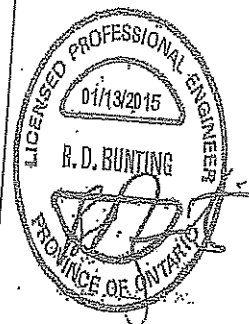
U.S. Patent
5,603,580



Double
Shear
Nailing
Side
View. Do
not bend
tab back.



Double
Shear
Nailing
Top View.



MADE
IN THE
UNITED STATES
DESIGN

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1.81/10 HUS210 1/15 Rev. 12/16

800-999-5099
www.strongtie.com

HGUS - Double Shear Joist Hangers

SIMPSON

Strong-Tie

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: 12 gauge

FINISH: G90 galvanized

DESIGN:

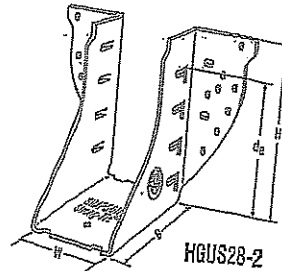
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

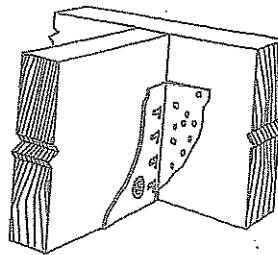
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

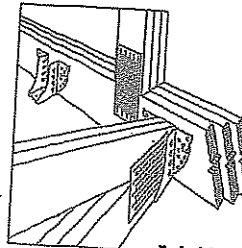
- See current catalogue for options.



HGUS28-2



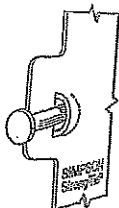
Typical HGUS Installation



Typical HGUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

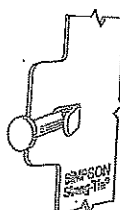
Model No.	Ga	Dimensions (in)					Fasteners		Factored Resistance (lbs)			
		W	H	B	d ₁	Face	Joist		D, Fir-L		S-P-F	
									Uplift	Normal	Uplift	Normal
HGUS26	12	1 1/2	5 1/2	5	4 1/2	20-16d	8-16d		2685	6625	2685	5700
HGUS26-2	12	3 1/2	6 1/2	4	4 1/2	20-16d	8-16d		4385	8950	3100	6355
HGUS26-3	12	4 1/2	5 1/2	4	4 1/2	20-16d	8-16d		4385	8950	3100	6355
HGUS26-4	12	6 1/2	5 1/2	4	4 1/2	20-16d	8-16d		4385	8950	3100	6355
HGUS28	12	1 1/2	7 1/2	5	6 1/2	30-16d	12-16d		3310	7675	3100	6900
HGUS28-2	12	3 1/2	7 1/2	4	6 1/2	30-16d	12-16d		6070	12980	4310	9215
HGUS28-3	12	4 1/2	7 1/2	4	6 1/2	30-16d	12-16d		6070	12980	4310	9215
HGUS28-4	12	6 1/2	7 1/2	4	6 1/2	30-16d	12-16d		6070	12980	4310	9215
HGUS210-2	12	3 1/2	8 1/2	4	8 1/2	46-16d	16-16d		6840	14645	4855	10400
HGUS210-3	12	4 1/2	8 1/2	4	8 1/2	46-16d	16-16d		6840	14645	4855	10400
HGUS210-4	12	6 1/2	8 1/2	4	8 1/2	46-16d	16-16d		6840	14645	4855	10400
HGUS212-4	12	6 1/2	10 1/2	4	10 1/2	58-16d	20-16d		7640	14995	5425	10845
HGUS214-4	12	6 1/2	12 1/2	4	11 1/2	66-16d	22-16d		10130	16400	7195	11845

1. d₂ is the distance from the seat of the hanger to the highest joist nail.

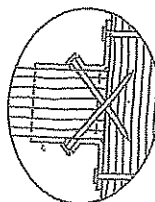


Double Shear Nailing prevents tabs breaking off (available on some models).

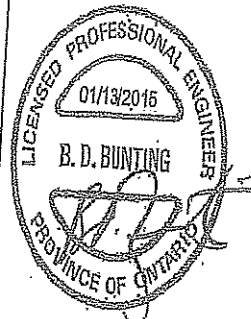
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



SIMPSON
STRONG-TIE
DESIGN

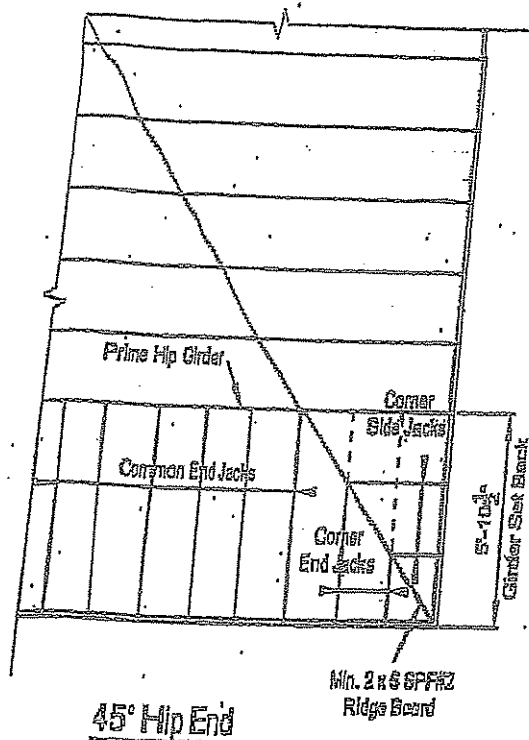
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800-999-5099
www.sbronline.com

CSPEC HGUS 15 1/16 12/16

MICRO CITY ENGINEERING SERVICES INC.

TEL: (519) 287-2242



LUMBER SPECIFICATION

TOP CHORD : 2x4 SPF#2

BOTTOM CHORD : 2x4 SPF#2

WEBS : 2x3 SPF#2

UNLESS OTHERWISE SHOWN

DESIGN LOAD:

TOP CHORD LIVE LOAD : 39.4 P.S.F.

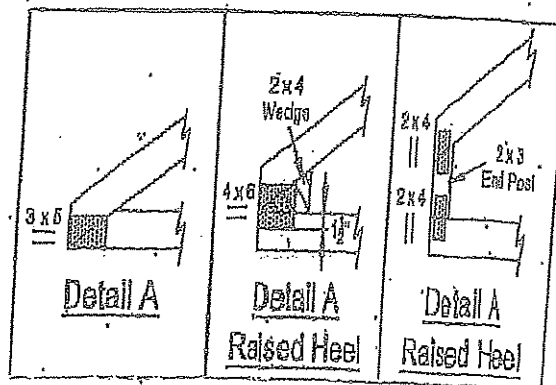
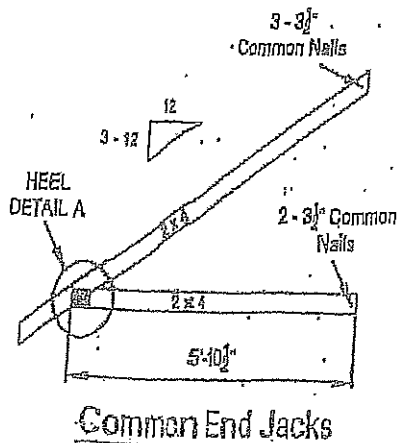
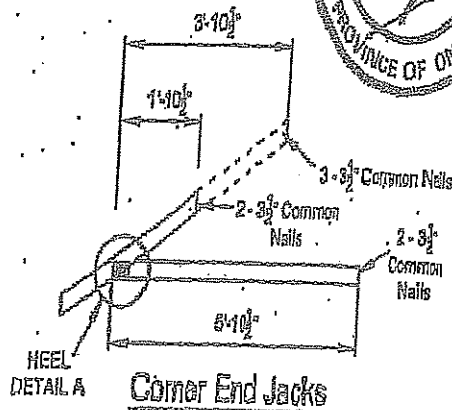
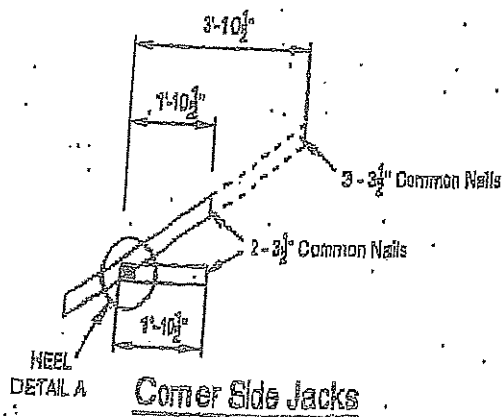
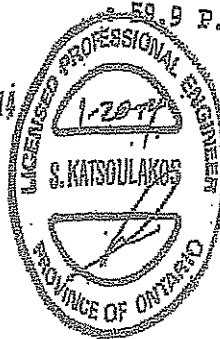
TOP CHORD DEAD LOAD : 3.0 P.S.F.

BOTTOM CHORD LIVE LOAD : 10.5 P.S.F.

BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD : 59.9 P.S.F.

DWG NO. TIM 2495.1A
STRUCTURAL
COMPONENT ONLY



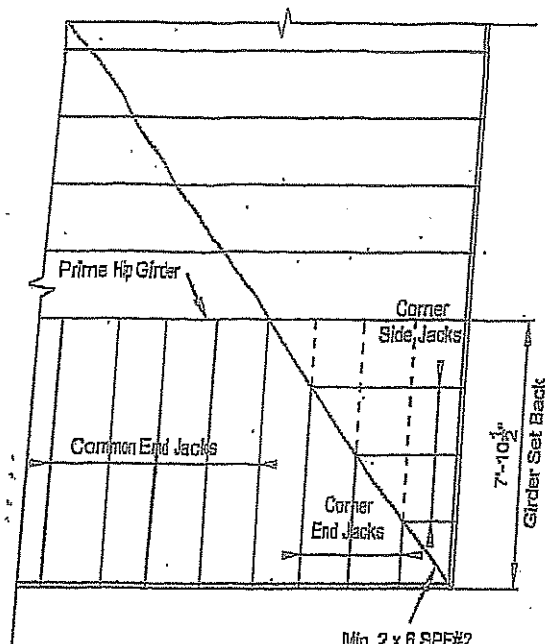
NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)

LUMBER SPECIFICATION

TOP CHORD : 2x4 SPF#2
 BOTTOM CHORD : 2x4 SPF#2
 WEBS : 2x3 SPF#2
 UNLESS OTHERWISE SHOWN

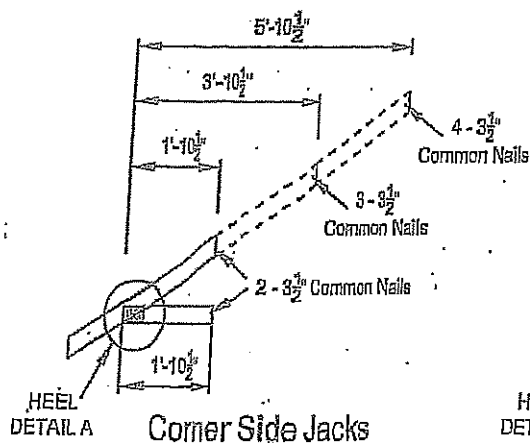
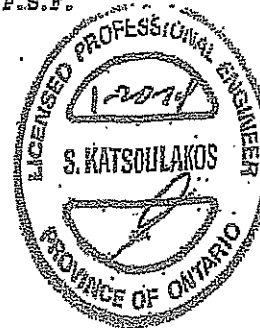
DESIGN LOAD:

TOP CHORD LIVE LOAD : 34.8 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.
 TOTAL LOAD : 44.8 P.S.F.

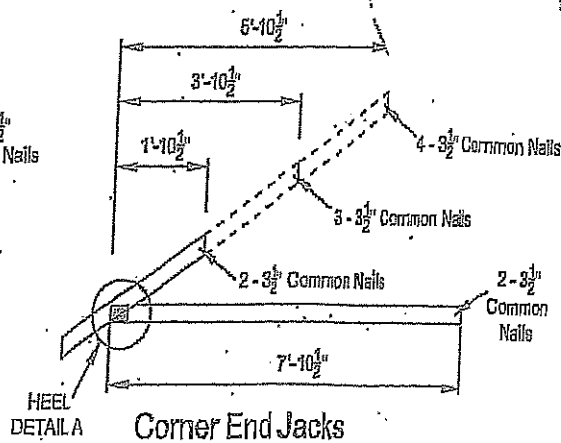


45° Hip End

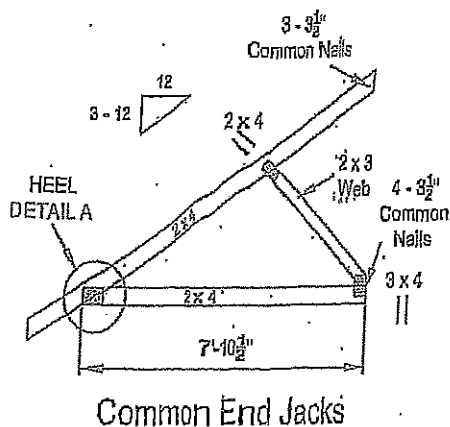
DWG NO T&M 3503.14
 STRUCTURAL
 COMPONENT ONLY



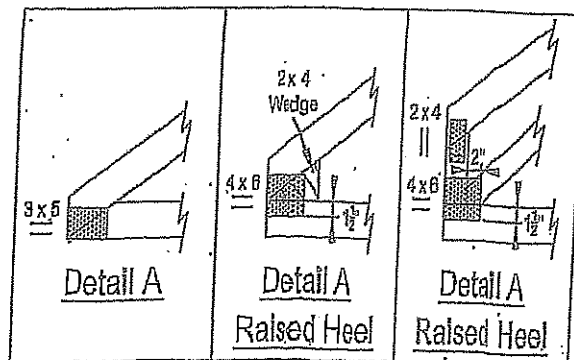
Corner Side Jacks



Corner End Jacks



Common End Jacks

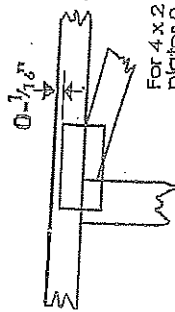


NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
 (TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

Symbols

PLATE LOCATION AND ORIENTATION

Center plate on joint unless x, y offsets are indicated.
Dimensions are in ft-in-sixteenths or mm.
Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/8\" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*Plate location details available in Mitek software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.



BEARING



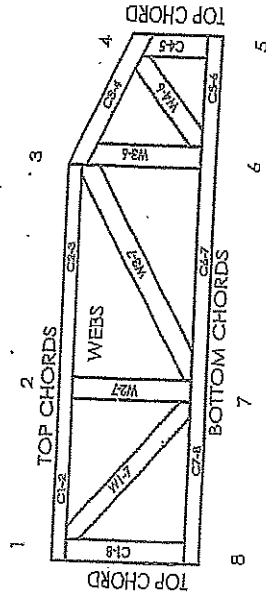
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

TPI-C: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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Mitek
POWER TO PERFORM™

Mitek Engineering Reference Sheet: MTK-7473C rev. 10-08



General Safety Notes

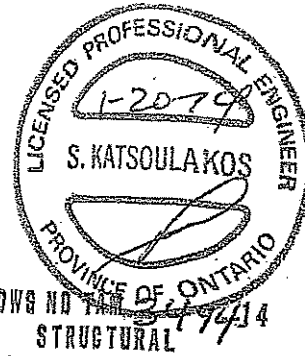
Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loadings shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPI-C.
7. Design assumes trusses will be suitably protected from the environment in accord with TPI-C.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPI-C Quality Criteria.

Micro City Engineering Services Inc.
(BCIN: 26064; FIRM BCIN: 29991)

RR #1, Po Box 61
Glencoe, Ontario
N0L 1M0

(519) 287 - 2242; Fax: (519) 287 - 5750 (Call)



Responsibilities:

Micro City Engineering Services is responsible for the design of trusses as individual components.

It is the responsibilities of others to ascertain that the design loads utilized on this (these) drawing(s) meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdiction over such decisions.

All dimensions are to be verified by the owner, contractor, architect, or other authority having input over such decisions prior to truss component manufacture. At no time shall Micro City Engineering Services Inc. or its employees be responsible for dimension errors.

Micro City Engineering Services Inc. bears no responsibility for the erection of any truss components. Persons erecting truss components are cautioned to seek professional advice regarding temporary and permanent bracing systems and to be totally familiar with all aspects of truss erection prior to proceeding on any truss component erection job. Any bracing shown on Micro City Engineering Services Inc. or Tamarack Roof Trusses Inc. sealed or unsealed truss component drawings is specified for the single truss component in question and is identified as an integral part of the design for that particular truss component but is not meant to represent the only required bracing for that particular truss component when installed as a component in a series of truss components in a roof truss system.

~~It is the truss manufacturer's responsibility to ensure that trusses are manufactured in accordance with Micro City Engineering Services Inc. specifications outlined below:~~

SPECIFICATIONS:

Truss components sealed by Micro City Engineering Services Inc. must conform to the relevant sections of the current Building Code of Ontario and Canada (Part 4 or Part 9) or the current Farm Building Code of Canada in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the Truss Plate Institute of Canada (TPIC). All unit lumber and nailing stresses identified on truss component design drawings and/or used in the design of individual truss components shall conform to the current CSA Wood Design standard identified in the current Building Code and TPIC Design Standards.

The lumber used to manufacture any truss component is to conform to the specified size and grade identified on the truss drawing.

The lumber used in the manufacture of any truss component is not to exceed 19% during its service use unless specifically noted on the truss drawing.

The lumber used in the manufacture of any truss component is not to be treated with any chemicals during its service life unless specifically noted on the truss drawing.

Connector plates shall be applied to both faces of the truss component at each joint and shall be positioned exactly as specified.

The top chord of any truss component is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the sealed truss component drawing but not exceeding 24" o/c (Part 9 design) and not exceeding 48" o/c (Part 4 or Agricultural design).

When a truss component is to be installed with no rigid ceiling attached directly to the bottom chord, then the bottom chord is to be laterally braced at intervals not exceeding 3m (or 10'-0").

All sealed or unsealed truss component drawings provided by Micro City Engineering Services Inc. Or Tamarack Roof Trusses Inc. should be read in conjunction with the following:

Warning-Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473C rev 10-'08 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer - not the truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpica.ca and BCSI Building Component Safety Information available from the Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA, 22314.