

ASPHALT SHINGLES
12" FINISH O.H.
R.T.M.C
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:

HGUS26-2 (XX)
LJS26DS (V)
LUS24 (O)

HIGHGROVE-5E
EL:2 UNIT1

286071

HIGHGROVE-3
EL:2 (REV)- UNIT2

286072

HIGHGROVE-3
EL:2 - UNIT3

286072

HIGHGROVE-4
EL:2 (REV)- UNIT4

286074

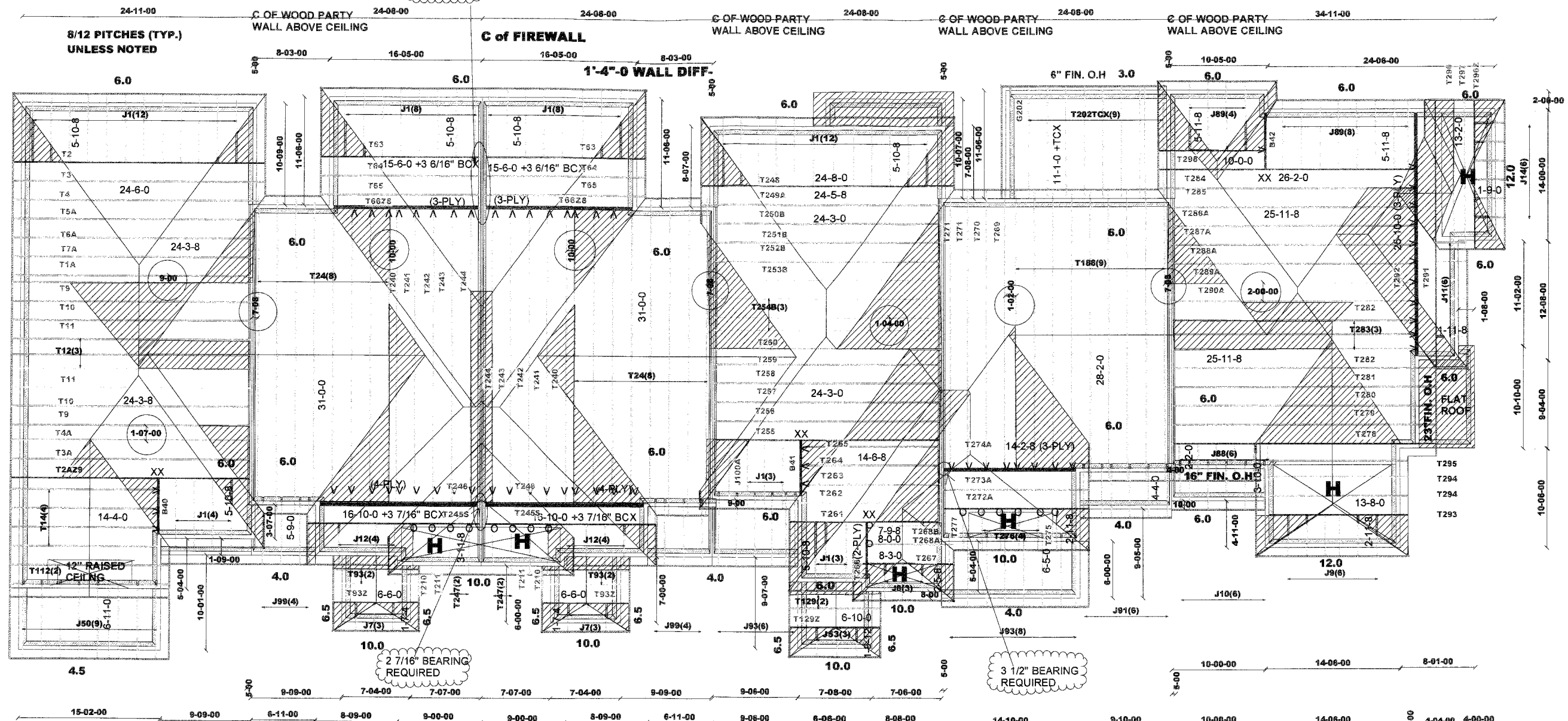
HIGHGROVE-1
EL:2 - UNIT5

286075

HIGHGROVE-11
EL:2 - UNIT6

286076

2 7/16" BEARING
REQUIRED



T-170534

DENOTES:
CONVENTIONAL
FRAMING



DENOTES:
H -2'-0-0 HIGHER PLATE

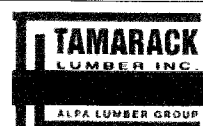
ALL B- 2-2X10 BEAMS (FLUSH)

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART 9 OF THE OBC LATEST EDITION
ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2\"X4\"SPF@24\"o.c.
WITH A 2\"X4\"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.
POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE
BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN LOADS:

SNOW LOAD 25.6 PSF
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

3 1/2\" BEARING
REQUIRED



Job Track: 43954
Layout ID: 283746
Plan Log: 93446

Builder / Location:

GREENPARK HOMES / WATERDOWN

Project: RUSSEL GARDENS PHASE 2

Date: 6/19/2018

Designer: sonny

Model / Elevation:

BLOCK218 / UNIT1TO6 (REVISED)

Mitek ver 7.5.0

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REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY
TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE



Delivery Shiplist

DATE	05/18/18
SALES REP	Mario

JOB TRACK:43954	LAYOUT ID: 286071	LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH	SUB-BUILDER:	
MODEL: HIGHGROVE 5E	ELEVATION: 2- UNIT1	

ROOF TRUSSES

ROOF TRUSS SPACING:24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	T1A COMMON	8.00 0.00	24-03-08	09-06-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	102.41 64.00		
	1	T2 HIP GIRDER	8.00 0.00	24-06-00	04-01-04	2 X 6	2 X 6	01-03-08 01-01-00	01-04-13 01-04-13	145.29 89.67		
	1	T2AZ9 HIP GIRDER	8.00 0.00	24-03-08	04-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-04-13 01-06-08	126.51 80.00		
	1	T3 HIP	8.00 0.00	24-06-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	97.09 60.83		
	1	T3A HIP	8.00 0.00	24-03-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	94.81 60.17		
	1	T4 HIP	8.00 0.00	24-06-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	105.79 67.00		
	1	T4A HIP	8.00 0.00	24-03-08	06-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	103.53 66.33		
	1	T5A HIP	8.00 0.00	24-03-08	07-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	104.32 67.00		
	1	T6A HIP	8.00 0.00	24-03-08	08-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	108.82 69.50		
	1	T7A HIP	8.00 0.00	24-03-08	09-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	114.67 72.50		
	2	T9 HALF HIP	8.00 0.00	24-03-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-01-04	197.10 123.66		
	2	T10 HALF HIP	8.00 0.00	24-03-08	06-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 06-01-04	212.66 133.34		
	2	T11 HALF HIP	8.00 0.00	24-03-08	07-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 07-01-04	225.56 142.68		
	3	T12 HALF HIP	8.00 0.00	24-03-08	08-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 08-01-04	351.96 219.51		
	4	T14 COMMON	8.00 0.00	14-04-00	06-02-02	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	247.04 156.00		
	2	T112 ROOF	8.00 0.00	14-04-00	06-02-02	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	135.80 89.00		
	16	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	268.64 170.72		
	9	J50 JACK	4.50 0.00	06-11-00	02-11-12	2 X 4	2 X 4	01-03-08 00-00-00	00-04-10 02-11-12	184.86 114.03		

TOTAL # TRUSS= 50

TOTAL BFT OF ALL TRUSSES=

1845.94 BFT. TOTAL WEIGHT OF ALL TRUSSES= 2926.86 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
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Delivery Shiplist

DATE	05/18/18
SALES REP	Mario

JOB TRACK:43954

LAYOUT ID: 286071

LOCATION:

BUILDER: GREENPARK - RUSSELL GARDENS PH

SUB-BUILDER:

MODEL: HIGHGROVE 5E

ELEVATION: 2- UNIT1

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	HGUS26-2	
3	Hangers	LJS26DS	

TOTAL # ITEMS= 4



Delivery Shiplist

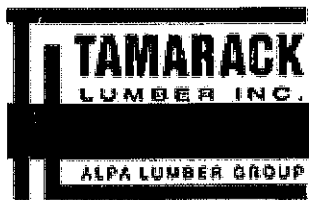
DATE	09/11/17
SALES REP	Mario

JOB TRACK: 43954	LAYOUT ID: 286072	LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH 2	SUB-BUILDER:	
MODEL: HIGHGROVE 3	ELEVATION: 2 (REV)-UNIT2 OR 2- UNIT3	

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	8	T24 COMMON	6.00 0.00	31-00-00	08-11-00	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	972.40 612.00		
	1	T240 ROOF	6.00 0.00	31-00-00	06-09-13	2 X 4	2 X 4	00-00-00 00-00-00	05-11-02 04-11-08	152.38 98.50		
	1	T241 ROOF	6.00 0.00	31-00-00	08-01-13	2 X 4	2 X 4	00-00-00 00-00-00	06-11-00 04-11-08	160.43 101.17		
	1	T242 ROOF	6.00 0.00	31-00-00	09-05-13	2 X 4	2 X 4	00-00-00 00-00-00	06-11-00 04-11-08	169.56 107.83		
	1	T243 ROOF	6.00 0.00	31-00-00	10-09-13	2 X 4	2 X 4	00-00-00 00-00-00	06-11-00 04-11-08	198.11 123.50		
	1	T244 ROOF	6.00 0.00	31-00-00	11-03-02	2 X 4	2 X 4	00-00-00 00-00-00	06-11-00 04-11-08	185.09 114.00		
	1	T245S ROOF	8.00 0.00	16-11-00	06-11-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 00-03-08	76.91 50.00		
	1 3 Ply	T246 HALF HIP	8.00 0.00	16-11-00	04-08-08	2 X 6	2 X 6	00-00-00 00-00-00	01-04-13 00-05-08	316.41 195.00		
	1	T63 HALF HIP	8.00 0.00	15-07-00	04-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-04-13 00-05-08	78.07 48.83		
	1	T64 HALF HIP	8.00 0.00	15-07-00	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 00-03-08	67.93 43.67		
	1	T65 HALF HIP	8.00 0.00	15-07-00	06-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 00-03-08	70.24 44.33		
	1 3 Ply	T66Z8 HALF HIP	8.00 0.00	15-07-00	06-08-12	2 X 6	2 X 6	00-00-00 00-00-00	01-04-13 00-05-08	330.21 198.00		
	1	T210 HALF HIP	10.00 0.00	03-11-08	02-02-13	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 04-02-13	21.65 14.67		
	1	T211 HALF HIP	10.00 0.00	03-11-08	03-06-13	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 05-06-13	26.04 17.67		
	2	T247 MONO TRUSS	10.00 0.00	03-11-08	04-11-04	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 06-11-04	48.08 32.00		
	2	T93 COMMON	6.50 0.00	06-06-00	02-04-09	2 X 4	2 X 4	01-03-08 01-03-08	00-07-07 00-07-07	44.40 30.00		
	1	T93Z COMMON	6.50 0.00	06-06-00	02-04-09	2 X 4	2 X 4	01-03-08 01-03-08	00-07-07 00-07-07	22.20 15.00		
	8	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	134.32 85.36		



Delivery Shiplist

DATE	09/11/17
SALES REP	Mario

JOB TRACK:43954 LAYOUT ID: 286072 LOCATION:
 BUILDER: GREENPARK - RUSSELL GARDENS PH SUB-BUILDER:
 MODEL: HIGHGROVE 3 ELEVATION: 2 (REV)-UNIT2 OR 2- UNIT3

ROOF TRUSSES

ROOF TRUSS SPACING:24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	3	J7 JACK-OPEN	10.00 0.00	01-07-04	02-04-07	2 X 4	2 X 4	01-03-08 00-00-00	01-00-06 02-04-07	21.27 15.99		
	4	J12 JACK-OPEN	8.00 0.00	02-11-08	03-04-08	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 03-04-08	42.04 26.68		
	4	J99 MONOPITCH	4.00 0.00	05-09-00	02-02-15	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-02-15	60.52 37.32		

TOTAL # TRUSS= 49.00 TOTAL BFT OF ALL TRUSSES= 2011.52 BFT. TOTAL WEIGHT OF ALL TRUSSES= 3198.26 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
4	Hangers	LUS24	
17	Hangers	LJS26DS	

TOTAL # ITEMS= 21.00



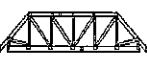
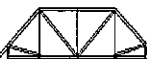
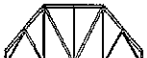
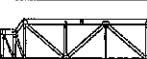
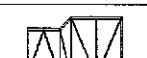
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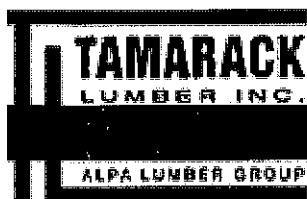
DATE	09/11/17
SALES REP	Mario

JOB TRACK:43954	LAYOUT ID: 286074	LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH 2	SUB-BUILDER:	
MODEL: HIGHGROVE4	ELEVATION: 2(REV)-UNIT4	

ROOF TRUSSES

ROOF TRUSS SPACING:24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	T248 HIP GIRDER	8.00 0.00	24-08-00	04-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-04-13 01-04-13	149.55 92.00		
	1	T249A HIP	8.00 0.00	24-05-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	95.30 60.17		
	1	T250B HIP	8.00 0.00	24-03-00	06-01-04	2 X 4	2 X 4	00-00-00 00-00-00	01-06-08 01-06-08	101.89 65.67		
	1	T251B HIP	8.00 0.00	24-03-00	07-01-04	2 X 4	2 X 4	00-00-00 00-00-00	01-06-08 01-06-08	102.64 66.33		
	1	T252B HIP	8.00 0.00	24-03-00	08-01-04	2 X 4	2 X 4	00-00-00 00-00-00	01-06-08 01-06-08	107.21 69.50		
	1	T253B HIP	8.00 0.00	24-03-00	09-01-04	2 X 4	2 X 4	00-00-00 00-00-00	01-06-08 01-06-08	113.01 71.83		
	3	T254B COMMON	8.00 0.00	24-03-00	09-07-08	2 X 4	2 X 4	00-00-00 00-00-00	01-06-08 01-06-08	304.41 191.49		
	1	T255 ROOF	8.00 0.00	24-03-00	04-01-04	2 X 6	2 X 6	00-00-00 00-00-00	03-01-12 04-01-04	141.56 91.00		
	1	T256 ROOF	8.00 0.00	24-03-00	05-01-04	2 X 4	2 X 4	00-00-00 00-00-00	04-01-12 05-01-04	104.50 66.00		
	1	T257 ROOF	8.00 0.00	24-03-00	06-01-04	2 X 4	2 X 4	00-00-00 00-00-00	05-01-12 06-01-04	112.94 70.83		
	1	T258 ROOF	8.00 0.00	24-03-00	07-01-04	2 X 4	2 X 4	00-00-00 00-00-00	06-01-12 07-01-04	125.33 79.00		
	1	T259 ROOF	8.00 0.00	24-03-00	08-01-04	2 X 4	2 X 4	00-00-00 00-00-00	07-01-12 08-01-04	130.69 81.67		
	1	T260 ROOF	8.00 0.00	24-03-00	08-11-00	2 X 4	2 X 4	00-00-00 00-00-00	07-01-12 08-11-00	134.01 84.50		
	1	T261 HALF HIP	8.00 0.00	14-06-08	04-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-04-13 04-01-04	76.23 46.67		
	1	T262 HALF HIP	8.00 0.00	14-06-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-01-04	64.38 41.00		
	1	T263 HALF HIP	8.00 0.00	14-06-08	06-01-04	2 X 4	2 X 4	00-00-00 00-00-00	01-04-13 06-01-04	65.03 41.00		
	1	T264 HALF HIP	8.00 0.00	14-06-08	07-01-04	2 X 4	2 X 4	00-00-00 00-00-00	01-04-13 07-01-04	65.10 40.67		
	1	T265 HALF HIP	8.00 0.00	14-06-08	08-01-04	2 X 4	2 X 4	00-00-00 00-00-00	01-04-13 08-01-04	71.05 45.00		



Delivery Shiplist

DATE	09/11/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 286074 LOCATION:
 BUILDER: GREENPARK - RUSSELL GARDENS PH 2 SUB-BUILDER:
 MODEL: HIGHGROVE4 ELEVATION: 2(REV)-UNIT4

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	T266 FLAT GIRDER	0.00	05-10-08	02-00-00	2 X 6	2 X 6	00-00-00	02-00-00	61.94		
	2 Ply		0.00					00-00-00	02-00-00	40.00		
	1	T267 HIP GIRDER	8.00 0.00	08-03-00	03-08-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	42.34 29.33		
	1	T268A COMMON	8.00 0.00	08-00-00	06-01-13	2 X 4	2 X 4	01-03-08 00-00-00	03-04-13 03-06-13	41.39 26.00		
	1	T268B COMMON	8.00 0.00	07-09-08	06-01-13	2 X 4	2 X 4	00-00-00 00-00-00	03-06-08 03-06-13	39.21 25.33		
	2	T129 COMMON	6.50 0.00	06-10-00	02-05-10	2 X 4	2 X 4	01-03-08 01-03-08	00-07-07 00-07-07	46.18 30.00		
	1	T129Z COMMON	6.50 0.00	06-10-00	02-05-10	2 X 4	2 X 4	01-03-08 01-03-08	00-07-07 00-07-07	23.09 15.00		
	18	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	302.22 192.06		
	3	J8 JACK-OPEN	10.00 0.00	02-05-08	03-08-04	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 03-08-04	30.09 21.99		
	3	J53 JACK-OPEN	10.00 0.00	01-08-12	02-05-11	2 X 4	2 X 4	01-03-08 00-00-00	01-00-06 02-05-11	22.26 15.99		
	6	J93 MONOPITCH	4.00 0.00	06-05-00	02-05-10	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-05-10	108.96 70.02		
	1	J100A JACK-OPEN	6.00 0.00	05-10-08	03-01-12	2 X 4	2 X 4	00-00-00 00-00-00	00-02-08 03-01-12	13.97 8.67		

TOTAL # TRUSS= 59.00

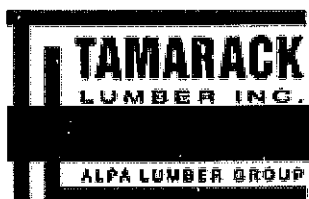
TOTAL BFT OF ALL TRUSSES=

1778.72 BFT. TOTAL WEIGHT OF ALL TRUSSES= 2796.48 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
3	Hangers	LJS26DS	
2	Hangers	LUS24	

TOTAL # ITEMS= 7.00



Delivery Shiplist

DATE	09/11/17
SALES REP	Mario

JOB TRACK:43954 LAYOUT ID: 286075 LOCATION:
 BUILDER: GREENPARK - RUSSELL GARDENS PH 2 SUB-BUILDER:
 MODEL: HIGHGROVE1 ELEVATION: 2-UNIT5

ROOF TRUSSES

ROOF TRUSS SPACING:24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	9	T188 COMMON	6.00 0.00	28-02-00	08-02-08	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	991.44 618.03		
	1	T269 HALF HIP	6.00 0.00	28-02-00	08-05-02	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 08-05-02	129.62 80.33		
	1	T270 HALF HIP	6.00 0.00	28-02-00	09-09-02	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 09-09-02	140.05 86.67		
	2	T271 COMMON	6.00 0.00	28-02-00	11-01-08	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 07-00-00	259.88 161.34		
	1	T272A HALF HIP	8.00 0.00	14-02-08	06-01-04	2 X 4	2 X 4	01-03-08 00-00-00	03-04-13 06-01-04	69.72 44.50		
	1	T273A HALF HIP	8.00 0.00	14-02-08	07-09-04	2 X 4	2 X 4	01-03-08 00-00-00	03-04-13 07-09-04	75.97 48.33		
	1 3 Ply	T274A HIP GIRDER	8.00 0.00	14-02-08	09-05-04	2 X 6	2 X 6	00-00-00 00-00-00	03-04-13 08-11-12	346.23 213.99		
	1	T275 HALF HIP	10.00 0.00	02-11-08	04-08-13	2 X 4	2 X 4	01-03-08 00-00-00	03-07-11 04-08-13	27.46 19.00		
	4	T276 MONO TRUSS	10.00 0.00	02-11-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 06-01-04	78.84 51.32		
	1	T277 MONO TRUSS	10.00 0.00	02-11-08	06-01-04	2 X 4	2 X 4	01-03-08 00-00-00	03-07-11 06-01-04	23.08 14.67		
	9	T202TCX MONOPITCH	3.00 0.00	11-01-00	03-07-13	2 X 6	2 X 4	00-09-08 00-00-00	00-08-04 03-07-13	431.37 279.00		
	1	G202 GABLE	3.00 0.00	11-01-00	03-05-08	2 X 6	2 X 4	00-09-08 00-00-00	00-08-04 03-05-08	43.96 29.33		
	6	J91 MONOPITCH	4.00 0.00	04-04-00	01-09-04	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 01-09-04	70.86 48.00		
	8	J93 MONOPITCH	4.00 0.00	06-05-00	02-05-10	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-05-10	145.28 93.36		

TOTAL # TRUSS= 48.00

TOTAL BFT OF ALL TRUSSES=

1787.87 BFT. TOTAL WEIGHT OF ALL TRUSSES= 2833.76 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
8	Hangers	LJS26DS	
6	Hangers	LUS24	

TOTAL # ITEMS= 14.00



Delivery Shiplist

DATE	04/17/18
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 286076 LOCATION:
 BUILDER: GREENPARK - RUSSELL GARDENS PH 2 SUB-BUILDER:
 MODEL: HIGHGROVE11 ELEVATION: 2-UNIT6

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	T278 HALF HIP	8.00 0.00	25-11-08	02-05-00	2 X 4	2 X 4	02-02-08 00-00-00	02-00-03 02-05-00	105.94 64.66		
	1	T279 HALF HIP	8.00 0.00	25-11-08	03-05-00	2 X 4	2 X 4	02-02-08 00-00-00	02-00-03 03-05-00	99.82 62.17		
	1	T280 HALF HIP	8.00 0.00	25-11-08	04-05-00	2 X 4	2 X 4	02-02-08 00-00-00	02-00-03 04-05-00	101.61 63.50		
	1	T281 HALF HIP	8.00 0.00	25-11-08	05-05-00	2 X 4	2 X 4	02-02-08 00-00-00	02-00-03 05-05-00	106.94 66.00		
	2	T282 HALF HIP	8.00 0.00	25-11-08	06-05-00	2 X 4	2 X 4	00-00-00 00-00-00	02-00-03 06-05-00	224.24 139.00		
	3	T283 HALF HIP	8.00 0.00	25-11-08	07-05-00	2 X 4	2 X 4	00-00-00 00-00-00	02-00-03 07-05-00	355.29 216.00		
	1	T284 HALF HIP	8.00 0.00	26-02-00	04-01-12	2 X 6	2 X 6	01-03-08 00-00-00	01-04-13 04-01-12	147.02 90.67		
	1	T285 HALF HIP	8.00 0.00	26-02-00	05-01-12	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-01-12	104.31 64.50		
	1	T286A HIP	8.00 0.00	25-11-08	06-01-12	2 X 4	2 X 4	00-00-00 00-00-00	01-06-08 02-00-03	105.25 66.50		
	1	T287A HIP	8.00 0.00	25-11-08	07-01-12	2 X 4	2 X 4	00-00-00 00-00-00	01-06-08 02-00-03	113.02 71.67		
	1	T288A HIP	8.00 0.00	25-11-08	08-01-12	2 X 4	2 X 4	00-00-00 00-00-00	01-06-08 02-00-03	117.70 75.33		
	1	T289A HIP	8.00 0.00	25-11-08	09-01-12	2 X 4	2 X 4	00-00-00 00-00-00	01-06-08 02-00-03	117.58 73.83		
	1	T290A HIP	8.00 0.00	25-11-08	10-02-00	2 X 4	2 X 4	00-00-00 00-00-00	01-06-08 02-00-03	123.81 78.17		
	1	T291 HIP GIRDER	6.00 0.00	25-10-00	02-08-08	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	94.95 62.50		
	1 3 Ply	T292 HIP GIRDER	6.00 0.00	25-10-00	04-00-08	2 X 4	2 X 6	00-00-00 00-00-00	01-02-00 01-02-00	397.44 252.00		
	1	T293 HIP GIRDER	8.00 0.00	13-08-00	04-10-00	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	62.01 39.00		
	2	T294 COMMON	8.00 0.00	13-08-00	05-11-08	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	118.52 74.00		
	1	T295 COMMON	8.00 0.00	13-08-00	07-11-08	2 X 4	2 X 4	01-03-08 01-03-08	03-04-13 03-04-13	70.51 43.67		



Delivery Shiplist

DATE	04/17/18
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 286076 LOCATION:
 BUILDER: GREENPARK - RUSSELL GARDENS PH 2 SUB-BUILDER:
 MODEL: HIGHGROVE11 ELEVATION: 2-UNIT6

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	T296 HIP GIRDER	6.00 0.00	13-02-00	03-07-08	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	54.63 37.00		
	1	T296Z HIP GIRDER	6.00 0.00	13-02-00	03-07-08	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	54.63 37.00		
	1	T297 COMMON	6.00 0.00	13-02-00	04-05-08	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	52.60 32.67		
	1	T298 HIP GIRDER	8.00 0.00	10-00-00	04-01-12	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-04-13	44.41 28.00		
	6	J88 JACK-OPEN	6.00 0.00	02-02-00	02-05-00	2 X 4	2 X 4	01-07-08 00-00-00	01-04-00 02-05-00	50.70 36.00		
	12	J89 JACK-OPEN	6.00 0.00	05-11-08	04-01-12	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-12	203.88 128.04		
	6	J9 JACK-OPEN	12.00 0.00	02-11-08	04-10-00	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 04-10-00	73.38 48.00		
	6	J10 MONOPITCH	6.00 0.00	03-10-00	02-05-12	2 X 4	2 X 4	01-03-08 00-00-00	00-06-12 02-05-12	70.44 43.98		
	6	J11 JACK-OPEN	8.00 0.00	01-11-08	02-08-08	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 02-08-08	47.94 31.98		
	6	J14 JACK-OPEN	12.00 0.00	01-09-00	03-07-08	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 03-07-08	53.40 36.00		

TOTAL # TRUSS= 70.00

TOTAL BFT OF ALL TRUSSES=

2061.84 BFT. TOTAL WEIGHT OF ALL TRUSSES= 3271.97 LBS.

HARDWARE

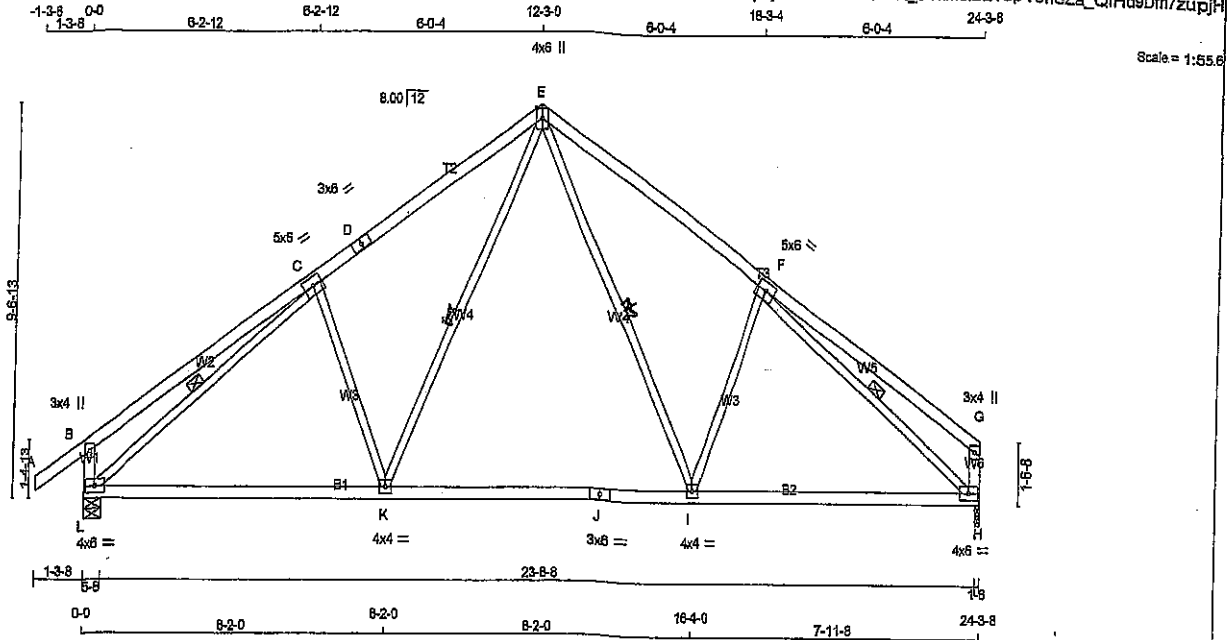
QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	HGUS26-2	
12	Hangers	LJS26DS	

TOTAL # ITEMS= 13.00

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
285994	T1A	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1:55.6

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
L - B	2x4	DRY	No.2
H - G	2x4	DRY	No.2
L - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE				
B TMV+p	MT20	3.0	4.0	
C TMWW-t	MT20	5.0	6.0	
D TS-t	MT20	3.0	8.0	
E TTWW+p	MT20	4.0	6.0	Edge
F TMV+p	MT20	5.0	6.0	
G TMV+p	MT20	3.0	4.0	
H BMVW-t	MT20	4.0	8.0	
I BMVW-t	MT20	4.0	4.0	
J BS-t	MT20	3.0	8.0	
K BMVW-t	MT20	4.0	4.0	
L BMVW-t	MT20	4.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
L	1479	0	1479	0	0	5-8	5-8	1-8	1-8
H	1393	0	1393	0	0	1-8	1-8		

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1198	692/0	255/0	0/0	0/0	251/0	0/0
H	1119	821/0	255/0	0/0	0/0	243/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-L, F-H. *E-K, E-J*

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO	CS (LC)	FR-TO	CS (LC)
A-B	0/32	-84.3 -84.3	0.11 (1)	C-K	-352/44
B-C	0/34	-84.3 -84.3	0.52 (1)	K-E	0/832
C-D	-1409/0	-84.3 -84.3	0.45 (1)	E-I	0/599
D-E	-1409/0	-84.3 -84.3	0.45 (1)	I-F	-316/58
E-F	-1388/0	-84.3 -84.3	0.44 (1)	F-H	-1689/0
F-G	0/34	-84.3 -84.3	0.50 (1)	H-I	-1653/0
L-B	-311/0	0.0 0.0	0.03 (1)		
H-G	-186/0	0.0 0.0	0.02 (1)		
L-K	0/1275	-28.0 -28.0	0.58 (2)		
K-J	0/889	-28.0 -28.0	0.54 (2)		
J-I	0/889	-28.0 -28.0	0.54 (2)		
I-H	0/1245	-28.0 -28.0	0.56 (2)		

TOTAL WEIGHT = 102 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.8 PSF
DL	= 3.0 PSF
BOT CH. LL	= 10.5 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)	= 1/360 (0.81")
CALCULATED VERT. DEFL.(LL)	= 1/999 (0.17")
ALLOWABLE DEFL.(TL)	= 1/360 (0.81")
CALCULATED VERT. DEFL.(TL)	= 1/999 (0.25")

CS: TC=0.52/1.00 (B-C:1), BC=0.58/1.00 (K-L:2), WB=0.71/1.00 (C-L:1), SSI=0.21/1.00 (C-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	818 354	1667 822

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (L) (INPUT = 0.90)
JSI METAL= 0.43 (C) (INPUT = 1.00)



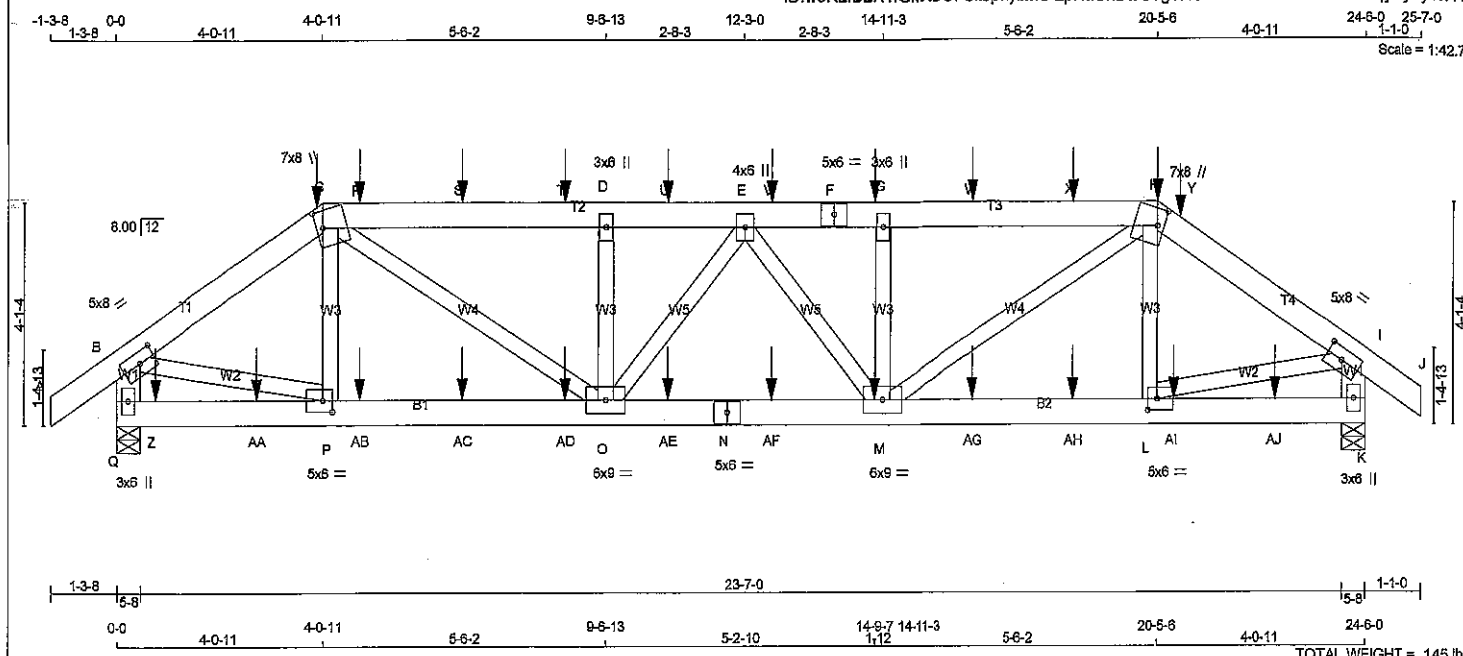
DRWG NO. TAW 3984-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	FLY	JOB DESC.	43954	DRWG NO.
283746	T2	1	1	TRUSS DESC.		

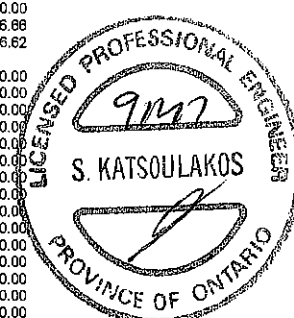
Tamarack Roof Truss, Burlington

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LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. A - C 2x6 DRY No.2 SPF C - F 2x6 DRY No.2 SPF F - H 2x6 DRY No.2 SPF H - J 2x6 DRY No.2 SPF K - B 2x6 DRY No.2 SPF K - I 2x6 DRY No.2 SPF Q - N 2x6 DRY No.2 SPF N - K 2x6 DRY No.2 SPF ALL WEBS 2x4 DRY No.2 SPF EXCEPT DRY: SEASONED LUMBER.				PLATES (table is in inches) JT TYPE PLATES W LEN Y X B TMVW-t MT20 5.0 8.0 2.50 3.75 C TTWW-m MT20 7.0 8.0 3.75 1.50 D TMW-w MT20 3.0 6.0 E TMWW-t MT20 4.0 8.0 F TS-t MT20 5.0 6.0 G TMW-w MT20 3.0 6.0 H TTWW-m MT20 7.0 8.0 3.75 1.50 I TMVW-t MT20 5.0 8.0 2.50 3.75 K BMV1-p MT20 3.0 6.0 L BMWW-t MT20 5.0 6.0 2.50 2.25 M BMWWW-t MT20 6.0 9.0 N BS-t MT20 5.0 6.0 O BMWWW-t MT20 6.0 9.0 P BMWW-t MT20 5.0 6.0 2.50 2.25 Q BMV1-p MT20 3.0 6.0				HANGERS NOTES 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 244.5 lbs FACTORED DOWN AT 4-0-11, 244.5 lbs FACTORED DOWN AT 20-5-5, 113.4 lbs FACTORED DOWN AT 4-9-4, 101.4 lbs FACTORED DOWN AT 6-9-4, 101.4 lbs FACTORED DOWN AT 8-9-4, 101.4 lbs FACTORED DOWN AT 10-9-4, 101.4 lbs FACTORED DOWN AT 12-9-4, 101.4 lbs FACTORED DOWN AT 14-9-4, 101.4 lbs FACTORED DOWN AT 16-9-4, AND 101.4 lbs FACTORED DOWN AT 18-9-4, AND 129.8 lbs FACTORED DOWN AT 20-9-4 ON TOP CHORD, AND 75.5 lbs FACTORED DOWN AT 9-4, 69.9 lbs FACTORED DOWN AT 2-9-4, 69.9 lbs FACTORED DOWN AT 4-9-4, 69.9 lbs FACTORED DOWN AT 6-9-4, 69.9 lbs FACTORED DOWN AT 8-9-4, 69.9 lbs FACTORED DOWN AT 10-9-4, 69.9 lbs FACTORED DOWN AT 12-9-4, 69.9 lbs FACTORED DOWN AT 14-9-4, 69.9 lbs FACTORED DOWN AT 16-9-4, 69.9 lbs FACTORED DOWN AT 18-9-4, AND 69.9 lbs FACTORED DOWN AT 20-9-4, AND 69.9 lbs FACTORED DOWN AT 22-9-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS FACTORED GROSS REACTION MAXIMUM FACTORED GROSS REACTION INPUT BRG REQRD BRG JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX Q 2441 0 2441 0 0 5-8 5-8 K 2455 0 2455 0 0 5-8 5-8 UNFACTORED REACTIONS 1ST LCASE MAX/MIN COMPONENT REACTIONS JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL Q 1997 1121 / 0 448 / 0 0 / 0 0 / 0 430 / 0 0 / 0 K 1994 1142 / 0 430 / 0 0 / 0 0 / 0 421 / 0 0 / 0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.96 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED. LOADING TOTAL LOAD CASES: (4) CHORDS FACTORED WEBS FACTORED MEMB. MAX. FORCE VERT. LOAD LC1 MAX. UNBRAC LENGTH FR-TO MEMB. MAX. FORCE VERT. LOAD LC1 MAX. UNBRAC LENGTH FR-TO (LBS) (PLF) CSI (LC) (LBS) CSI (LC) FR-TO FROM TO A-B 0 / 33 -84.3 -84.3 0.07 (1) 10.00 P-C -234 / 201 0.04 (1) B-C -2812 / 0 -84.3 -84.3 0.17 (1) 4.82 C-O 0 / 1872 0.33 (1) C-R -3889 / 0 -84.3 -84.3 0.40 (1) 3.96 O-D -720 / 0 0.12 (1) R-S -3889 / 0 -84.3 -84.3 0.40 (1) 3.96 M-G -717 / 0 0.12 (1) S-T -3889 / 0 -84.3 -84.3 0.40 (1) 3.96 M-H 0 / 1835 0.32 (1) T-D -3889 / 0 -84.3 -84.3 0.40 (1) 3.96 L-H -247 / 197 0.04 (1) D-U -3889 / 0 -84.3 -84.3 0.31 (1) 4.08 B-P 0 / 2387 0.42 (1) U-E -3889 / 0 -84.3 -84.3 0.29 (1) 4.08 L-I 0 / 2434 0.43 (1) E-V -3903 / 0 -84.3 -84.3 0.29 (1) 4.08 O-E -77 / 0 0.02 (1) V-F -3903 / 0 -84.3 -84.3 0.29 (1) 4.08 E-M -52 / 0 0.01 (1) F-G -3903 / 0 -84.3 -84.3 0.29 (1) 4.03 G-W -3903 / 0 -84.3 -84.3 0.39 (1) 3.97 W-X -3903 / 0 -84.3 -84.3 0.39 (1) 3.97 X-H -3903 / 0 -84.3 -84.3 0.39 (1) 3.97 H-Y -2838 / 0 -84.3 -84.3 0.19 (1) 4.77 Y-I -2838 / 0 -84.3 -84.3 0.19 (1) 4.77 I-J 0 / 28 -84.3 -84.3 0.05 (1) 10.00 O-B -2358 / 0 0.0 0.0 0.17 (1) 6.86 K-I -2391 / 0 0.0 0.0 0.17 (1) 6.82 Q-Z 0 / 0 -28.0 -28.0 0.14 (3) 10.00 Z-AA 0 / 0 -28.0 -28.0 0.14 (3) 10.00 AA-P 0 / 0 -28.0 -28.0 0.14 (3) 10.00 P-AB 0 / 2325 -28.0 -28.0 0.38 (1) 10.00 AB-AC 0 / 2325 -28.0 -28.0 0.38 (1) 10.00 AC-AD 0 / 2325 -28.0 -28.0 0.38 (1) 10.00 AD-O 0 / 2325 -28.0 -28.0 0.38 (1) 10.00 O-AE 0 / 3934 -28.0 -28.0 0.60 (1) 10.00 AE-N 0 / 3934 -28.0 -28.0 0.60 (1) 10.00 N-AF 0 / 3934 -28.0 -28.0 0.60 (1) 10.00 AF-M 0 / 3934 -28.0 -28.0 0.60 (1) 10.00 M-AG 0 / 2370 -28.0 -28.0 0.38 (1) 10.00 AG-AH 0 / 2370 -28.0 -28.0 0.38 (1) 10.00 AH-L 0 / 2370 -28.0 -28.0 0.38 (1) 10.00 L-AJ 0 / 0 -28.0 -28.0 0.14 (3) 10.00 AJ-AK 0 / 0 -28.0 -28.0 0.14 (3) 10.00 AK-K 0 / 0 -28.0 -28.0 0.14 (3) 10.00				DESIGN CRITERIA *** SPECIAL LOADS ANALYSIS *** GEOMETRY AND/OR BASIC LOADS CHANGED BY USER LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE SPECIFIED LOADS: TOP CH. LL = 25.6 PSF DL = 3.0 PSF BOT CH. LL = 10.5 PSF DL = 7.0 PSF TOTAL LOAD = 46.1 PSF SPACING = 24.0 IN/C LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12 *** NON STANDARD GIRDER *** ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES. THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010 THIS DESIGN COMPLIES WITH: - PART 9 OF CBC 2012, BCBC 2012, ABC 2014 - CSA 086-09 - TPIC 2011 (55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL)= L/360 (0.82") CALCULATED VERT. DEFL.(LL) = U/999 (0.13") ALLOWABLE DEFL.(TL)= L/360 (0.82") CALCULATED VERT. DEFL.(TL) = U/999 (0.20") CSI: TC=0.40 (C-D:1), BC=0.60 (M-O:1), WB=0.43 (L-L:1), SSI=0.29 (C-D:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00 COMPANION LIVE LOAD FACTOR = 0.50 AUTOSOLVE HEELS OFF TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PL) (FLI) MAX MIN MAX MIN MAX MIN MT20 618 354 1667 822 2284 1658 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.88 (L) (INPUT = 0.90) JSI METAL= 0.88 (N) (INPUT = 1.00)			
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DWG NO. TAM 46056-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
283746	T2	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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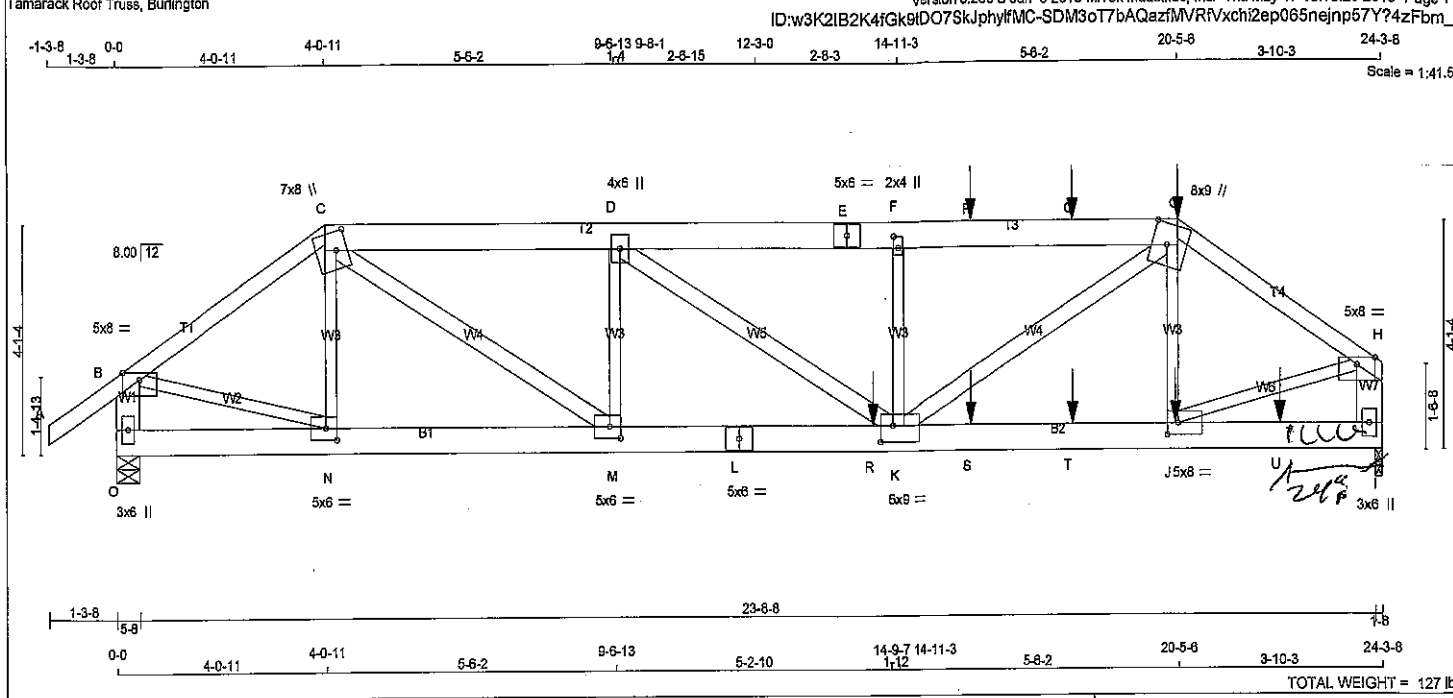
FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	4-0-11	-23	-25	—	FRONT	VERT	DEAD
C	4-0-11	-222	-222	—	FRONT	VERT	SNOW
G	14-9-4	-101	-101	—	BACK	VERT	TOTAL
H	20-5-5	-23	-25	—	FRONT	VERT	DEAD
H	20-5-5	-222	-222	—	FRONT	VERT	SNOW
M	14-9-4	-40	-70	—	BACK	VERT	TOTAL
R	4-9-4	-113	-113	—	BACK	VERT	TOTAL
S	8-9-4	-101	-101	—	BACK	VERT	TOTAL
T	8-9-4	-101	-101	—	BACK	VERT	TOTAL
U	10-9-4	-101	-101	—	BACK	VERT	TOTAL
V	12-9-4	-101	-101	—	BACK	VERT	TOTAL
W	16-9-4	-101	-101	—	BACK	VERT	TOTAL
X	18-9-4	-101	-101	—	BACK	VERT	TOTAL
Y	20-9-4	-130	-130	—	BACK	VERT	TOTAL
Z	9-4	-43	-76	—	BACK	VERT	TOTAL
AA	2-9-4	-40	-70	—	BACK	VERT	TOTAL
AB	4-9-4	-40	-70	—	BACK	VERT	TOTAL
AC	6-9-4	-40	-70	—	BACK	VERT	TOTAL
AD	8-9-4	-40	-70	—	BACK	VERT	TOTAL
AE	10-9-4	-40	-70	—	BACK	VERT	TOTAL
AF	12-9-4	-40	-70	—	BACK	VERT	TOTAL
AG	16-9-4	-40	-70	—	BACK	VERT	TOTAL
AH	18-9-4	-40	-70	—	BACK	VERT	TOTAL
AI	20-9-4	-40	-70	—	BACK	VERT	TOTAL
AJ	22-9-4	-40	-70	—	BACK	VERT	TOTAL



DWG NO. TAM 46056-17
STRUCTURAL
COMPONENT ONLY

P022



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - E	2x6	DRY No.2	SPF
E - G	2x6	DRY No.2	SPF
G - H	2x4	DRY No.2	SPF
O - B	2x6	DRY No.2	SPF
I - H	2x6	DRY No.2	SPF
O - L	2x6	DRY No.2	SPF
L - I	2x6	DRY No.2	SPF
ALL WEBS	2x3	DRY No.2	SPF
EXCEPT			
C - M	2x4	DRY No.2	SPF
K - G	2x4	DRY No.2	SPF
D - K	2x4	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	7.0	8.0	4.00	2.50
D	TMWW+t	MT20	4.0	6.0		
E	TS-t	MT20	5.0	6.0		
F	TMW+w	MT20	2.0	4.0	2.50	1.00
G	TTWW+m	MT20	8.0	9.0	4.50	3.50
H	TMVW-p	MT20	5.0	8.0	Edge	
I	BMV1+p	MT20	3.0	6.0		
J	BMVW+t	MT20	5.0	8.0	2.50	2.50
K	BMVW+t	MT20	8.0	9.0	3.50	2.75
L	BS-t	MT20	5.0	6.0		
M	BMVW+t	MT20	5.0	6.0	2.50	2.50
N	BMVW+t	MT20	5.0	6.0	2.50	2.50
O	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 101.4 lbs FACTORED DOWN AT 18-4-12, AND 101.4 lbs FACTORED DOWN AT 18-4-12, AND 349.5 lbs FACTORED DOWN AT 20-5-6 ON TOP CHORD, AND 1349.4 lbs FACTORED DOWN AT 14-5-8, 69.9 lbs FACTORED DOWN AT 16-4-12, 69.9 lbs FACTORED DOWN AT 18-4-12, AND 69.9 lbs FACTORED DOWN AT 20-4-12, AND 69.9 lbs FACTORED DOWN AT 22-4-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

SCAB JOINT # 1 WITH 2 X 6 #2 SPF 24" LONG 1 SIDE(S) USING 2 ROW(S) 3/4" ARDOX SPIRAL COMMON WIRE NAILS AT 2" O/C STAGGERED (16 NAILS/SCAB). REQ JIPED FOR 1/2" BRG PROVISION.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	MAXIMUM FACTORED GROSS REACTION UP	INPUT BRG IN-SX	REQ'D BRG IN-SX
O	2171	0	2171	0	5-8	5-8
I	2733	0	2733	0	1-8	1-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1759	1014 / 0	375 / 0	0 / 0	0 / 0	370 / 0	0 / 0
I	2230	1260 / 0	492 / 0	0 / 0	0 / 0	478 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.48 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 32	-84.3 -84.3	0.12 (1)	10.00	N-C	-357 / 3	0.09 (1)
B-C	-2438 / 0	-84.3 -84.3	0.39 (1)	4.00	C-M	0 / 2449	0.43 (1)
C-D	-4063 / 0	-84.3 -84.3	0.29 (1)	4.02	M-D	-1091 / 0	0.26 (1)
D-E	-4931 / 0	-84.3 -84.3	0.33 (1)	3.65	K-F	-843 / 0	0.15 (1)
E-F	-4931 / 0	-84.3 -84.3	0.33 (1)	3.65	K-G	0 / 2771	0.49 (1)
F-P	-4932 / 0	-84.3 -84.3	0.47 (1)	3.49	J-G	-565 / 42	0.14 (1)
P-Q	-4932 / 0	-84.3 -84.3	0.47 (1)	3.49	B-N	0 / 2093	0.52 (1)
Q-G	-4932 / 0	-84.3 -84.3	0.47 (1)	3.49	J-H	0 / 2748	0.68 (1)
G-H	-3164 / 0	-84.3 -84.3	0.42 (1)	3.53	D-K	0 / 1050	0.19 (1)
O-B	-2117 / 0	0.0	0.0	1.5			
I-H	-2663 / 0	0.0	0.0	0.19 (1)			
O-N	0 / 0	-28.0 -28.0	0.07 (3)	10.00			
N-M	0 / 2025	-28.0 -28.0	0.35 (1)	10.00			
M-L	0 / 4063	-28.0 -28.0	0.82 (1)	10.00			
L-R	0 / 4063	-28.0 -28.0	0.62 (1)	10.00			
R-K	0 / 4063	-28.0 -28.0	0.82 (1)	10.00			
K-S	0 / 2625	-28.0 -28.0	0.51 (1)	10.00			
S-T	0 / 2825	-28.0 -28.0	0.51 (1)	10.00			
T-J	0 / 2825	-28.0 -28.0	0.51 (1)	10.00			
J-U	0 / 0	-28.0 -28.0	0.10 (3)	10.00			
U-I	0 / 0	-28.0 -28.0	0.10 (3)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYP.
G	20-5-6	-105	-105		FRONT	VERT	TOTAL
G	20-5-6	-23	-25		FRONT	VERT	TOTAL
G	20-5-6	-222	-222		FRONT	VERT	TOTAL
	20-4-12	-40	-70		FRONT	VERT	TOTAL
	18-4-12	-101	-101		FRONT	VERT	TOTAL
	18-4-12	-101	-101		FRONT	VERT	TOTAL
	14-5-8	-1349	-1349		FRONT	VERT	TOTAL
	16-4-12	-40	-70		FRONT	VERT	TOTAL
	18-4-12	-40	-70		FRONT	VERT	TOTAL
	22-4-12	-40	-70		FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDTL. USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC0 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/999 (0.17")
ALLOWABLE DEFL.(TL) = L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/999 (0.28")

CSI: TC=0.47/1.00 (F-G-1), BC=0.82/1.00 (K-M-1), WB=0.68/1.00 (H-J-1), SS=0.86/1.00 (K-M-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	618	354	1687
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JOB NAME 286071	TRUSS NAME T2AZ9	QUANTITY 1	PLY 1	JOB DESC. 43954 TRUSS DESC.	DRWG NO.
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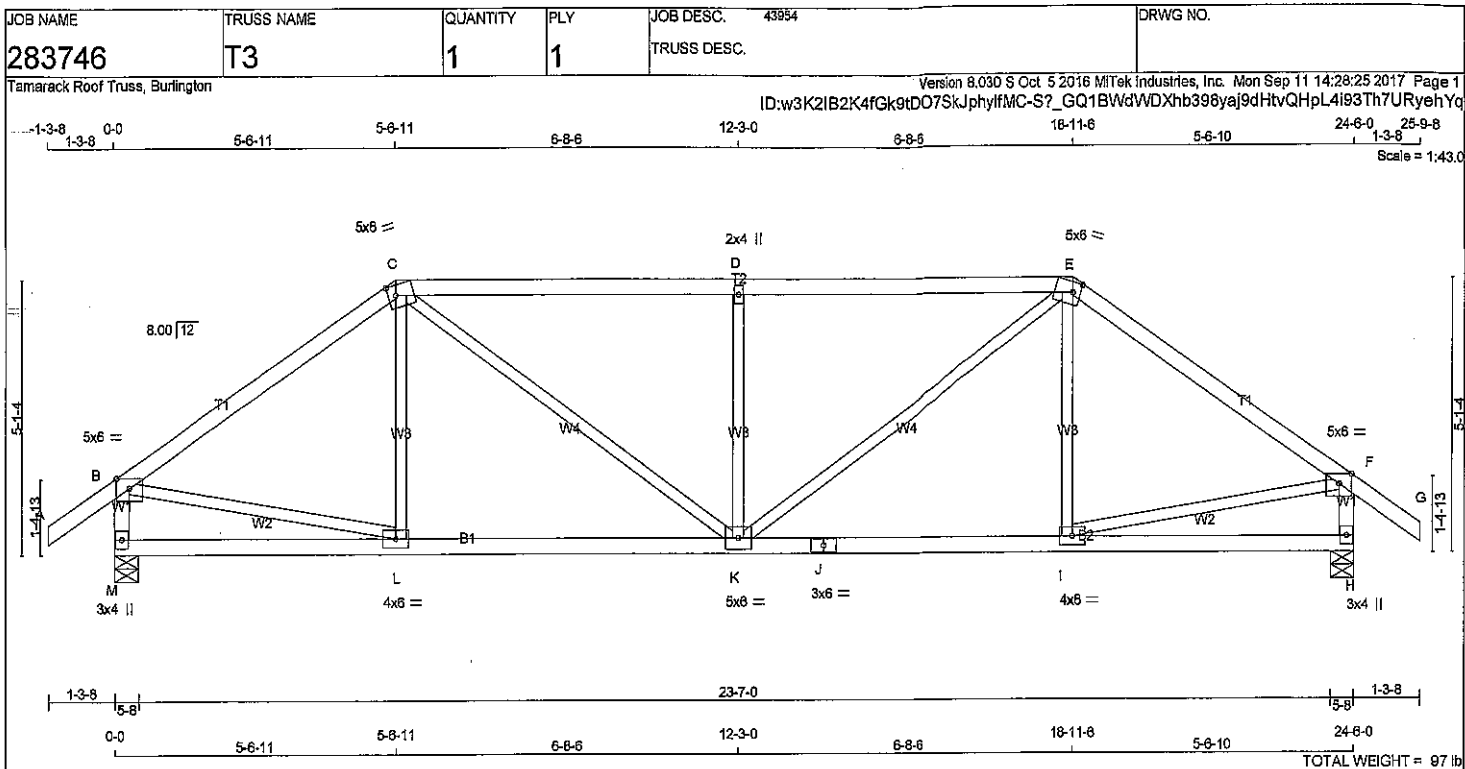
Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MTek Industries, Inc. Thu May 17 13:10:23 2018 Page 2
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JSI GRIP= 0.89 (J) (INPUT = 0.90)
JSI METAL= 0.78 (L) (INPUT = 1.00)



part
DWG NO. TAM26009-18
STRUCTURAL
COMPONENT ONLY



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
M - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
M - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVV-p	MT20	5.0	6.0	Edge	
C	TTWW-m	MT20	5.0	6.0	2.25	1.75
D	TMW-w	MT20	2.0	4.0		
E	TTWW-m	MT20	5.0	6.0	2.25	1.75
F	TMVV-p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
M	1491	0	1491	0	5-8	5-8
H	1491	0	1491	0	5-8	5-8

UNFACTORED REACTIONS

JT	MAX./MIN. COMPONENT REACTIONS						
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
M	1208	697 / 0	257 / 0	0 / 0	0 / 0	253 / 0	0 / 0
H	1208	697 / 0	257 / 0	0 / 0	0 / 0	253 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.85 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

FR-TO	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. UNBRACED LENGTH (FT)	MAX. FACTORED FORCE (LBS)
A-B	0 / 32	-84.3 -84.3 0.11 (1)	10.00	L-C -47 / 210 0.05 (3)
B-C	-1511 / 0	-84.3 -84.3 0.55 (1)	4.67	C-K 0 / 747 0.17 (1)
C-D	-1862 / 0	-84.3 -84.3 0.80 (1)	3.85	K-D -692 / 0 0.27 (1)
D-E	-1862 / 0	-84.3 -84.3 0.80 (1)	3.85	E-K 0 / 747 0.17 (1)
E-F	-1511 / 0	-84.3 -84.3 0.55 (1)	4.67	F-E -47 / 210 0.05 (3)
F-G	0 / 32	-84.3 -84.3 0.11 (1)	10.00	B-L 0 / 1278 0.28 (1)
M-B	-1430 / 0	0.0 0.0 0.15 (1)	6.83	L-F 0 / 1278 0.28 (1)
H-F	-1430 / 0	0.0 0.0 0.15 (1)	6.83	
M-L	0 / 0	-28.0 -28.0 0.25 (3)	10.00	
L-K	0 / 1255	-28.0 -28.0 0.40 (2)	10.00	
K-J	0 / 1255	-28.0 -28.0 0.40 (2)	10.00	
J-I	0 / 1255	-28.0 -28.0 0.40 (2)	10.00	
I-H	0 / 0	-28.0 -28.0 0.25 (3)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
	DL = 3.0 PSF
BOT CH.	LL = 10.5 PSF
	DL = 7.0 PSF
TOTAL LOAD	= 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.82")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.08")
ALLOWABLE DEFL.(TL) = $L/360$ (0.82")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.14")

CSI: TC=0.80 (C-D:1), BC=0.40 (K-L:2), WB=0.29 (B-L:1), CSI=0.27 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1657 822	2284 1856

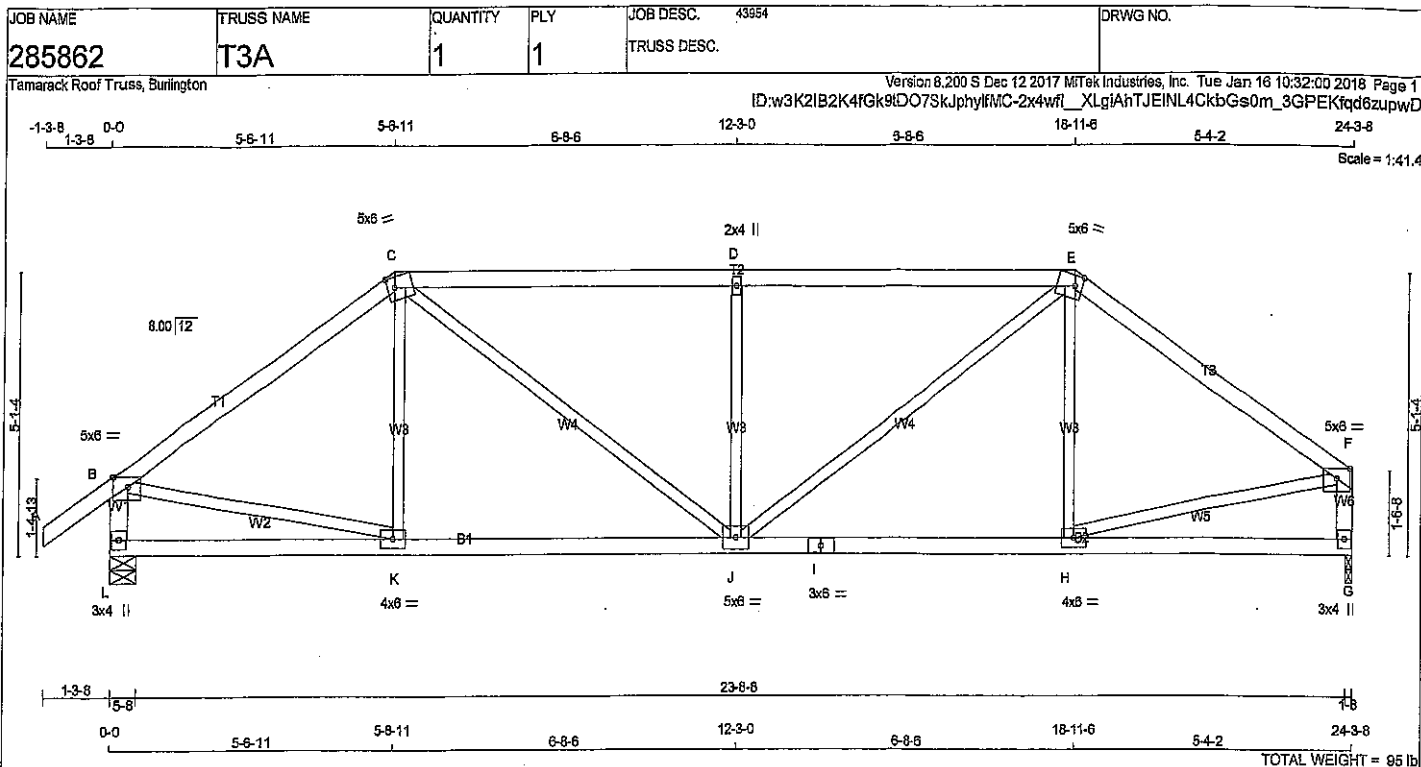
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (L) (INPUT = 0.90)
JSI METAL= 0.37 (J) (INPUT = 1.00)



DWNO.YAN46058-17
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
L - B	2x4	DRY	No.2
G - F	2x4	DRY	No.2
L - I	2x4	DRY	No.2
I - G	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TTVW-m	MT20	5.0	6.0	2.25	1.75
D	TMVW-w	MT20	2.0	4.0		
E	TTVW-m	MT20	5.0	6.0	2.25	1.75
F	TMVW-p	MT20	5.0	6.0	Edge	
G	BMV1-p	MT20	3.0	4.0		
H	BMVW-t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMVW-t	MT20	5.0	6.0		
K	BMVW-t	MT20	4.0	6.0		
L	BMV1-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
L	1479	0	1479	0	5-8	5-8
G	1363	0	1363	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	WIND			
L	1198	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0
G	1119	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

CHORDS				WEBS				
MAX. FACTORED		FACTORED		MAX. FACTORED				
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MAX. UNBRAC	MEMB.	FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		LENGTH	FR-TO			
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	K-C	-45 / 211	0.05 (3)
B-C	-1495 / 0	-84.3	-84.3	0.55 (1)	4.69	C-J	0 / 728	0.16 (1)
C-D	-1833 / 0	-84.3	-84.3	0.80 (1)	3.68	J-D	-692 / 0	0.27 (1)
D-E	-1833 / 0	-84.3	-84.3	0.80 (1)	3.68	J-E	0 / 769	0.17 (1)
E-F	-1458 / 0	-84.3	-84.3	0.50 (1)	4.81	H-E	-81 / 187	0.04 (3)
L-B	-1418 / 0	0.0	0.0	0.15 (1)	8.85	B-K	0 / 1265	0.28 (1)
G-F	-1305 / 0	0.0	0.0	0.14 (1)	7.05	H-F	0 / 1239	0.28 (1)
L-K	0 / 0	-28.0	-28.0	0.25 (3)	10.00			
K-J	0 / 1241	-28.0	-28.0	0.40 (2)	10.00			
J-I	0 / 1208	-28.0	-28.0	0.39 (2)	10.00			
I-H	0 / 1208	-28.0	-28.0	0.39 (2)	10.00			
H-G	0 / 0	-28.0	-28.0	0.24 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 068-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.81")
CALCULATED VERT. DEFL. (LL) = L/999 (0.08")
ALLOWABLE DEFL. (TL) = L/360 (0.81")
CALCULATED VERT. DEFL. (TL) = L/999 (0.14")

CS1: TC=0.80/1.00 (D-E:1), BC=0.40/1.00 (J-K:2), WB=0.28/1.00 (B-K:1), SS=0.27/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)		SHEAR (PSI)	SECTION (PLI)	
	(PSI)	(PLI)		(PLI)	(PLI)
MT20	618	354	1867	822	2284

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.66 (E) (INPUT = 0.80)
JSI METAL= 0.36 (I) (INPUT = 1.00)



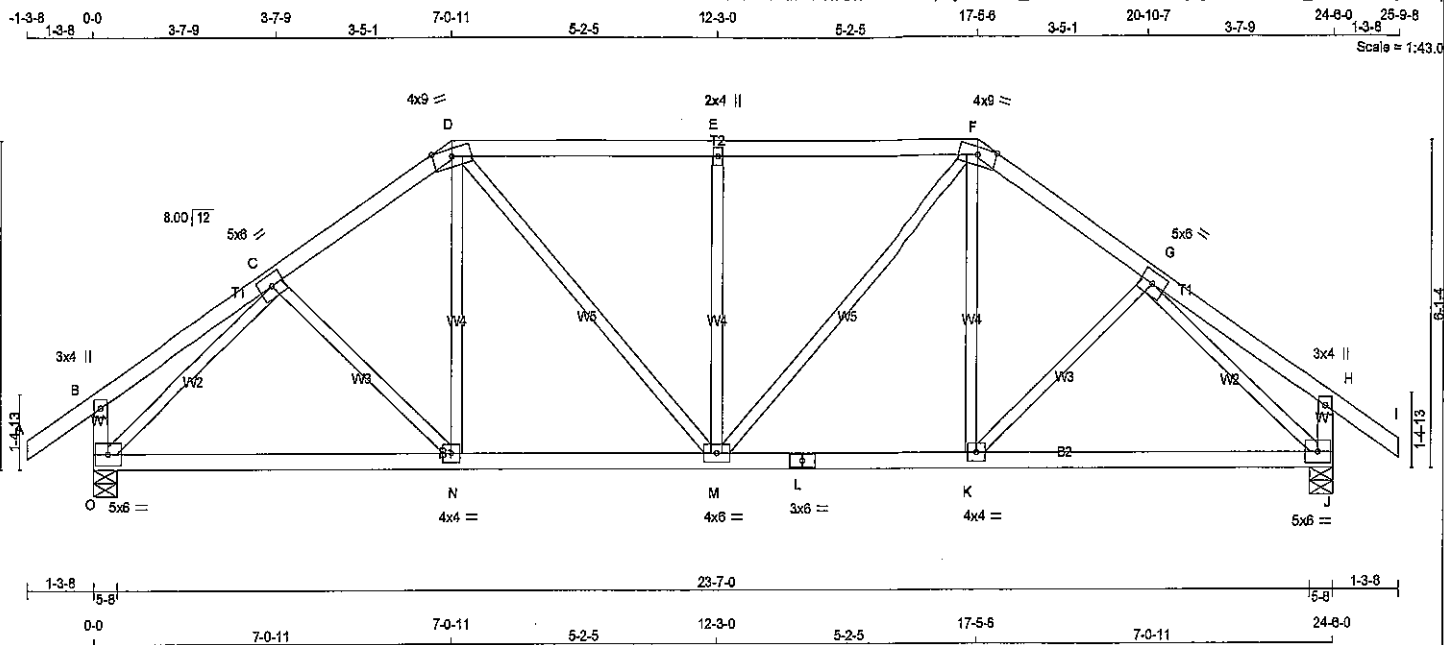
DWG NO. TAM 4008-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43854	DRWG NO.
283746	T4	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	FACTORED
D - F	2x4	DRY	No.2	SPF	GROSS REACTION
F - I	2x4	DRY	No.2	SPF	JT VERT
O - B	2x4	DRY	No.2	SPF	HORZ
J - H	2x4	DRY	No.2	SPF	DOWN
O - L	2x4	DRY	No.2	SPF	HORZ
L - J	2x4	DRY	No.2	SPF	UPLIFT
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	IN-SX

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	3.0	4.0			
C TMW+t	MT20	5.0	6.0			
D TTVW+m	MT20	4.0	9.0	Edge		
E TMW+w	MT20	2.0	4.0			
F TTVW+m	MT20	4.0	9.0	Edge		
G TMW+w	MT20	5.0	6.0			
H TMV+p	MT20	3.0	4.0			
J BMVW1-t	MT20	5.0	6.0			
K BMWV-t	MT20	4.0	4.0			
L BS-t	MT20	3.0	6.0			
M BMWVW-t	MT20	4.0	6.0			
N BMWVW-t	MT20	4.0	4.0			
O BMVW1-t	MT20	5.0	6.0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
O	1491	0	1491	0
J	1491	0	1491	0

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1208	697 / 0	257 / 0	0 / 0	0 / 0	253 / 0	0 / 0
J	1208	697 / 0	257 / 0	0 / 0	0 / 0	253 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.02 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT, OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	MAX. FACTORED	WEBS	MAX. FACTORED	MAX. FACTORED	MAX. FACTORED
MEMB.	FORCE	VERT. LOAD LC1	MAX	MEMB.	FORCE	MAX	MAX
	(LBS)	(PLF)	CSI (LC)		(LBS)	CSI (LC)	CSI (LC)
FR-TO		FROM TO	LENGTH FR-TO	FR-TO			
A-B	0 / 32	-84.3	-84.3 0.11 (1)	C-N	0 / 119	0.03 (3)	
B-C	0 / 17	-84.3	-84.3 0.15 (1)	N-D	0 / 304	0.07 (2)	
C-D	-1476 / 0	-84.3	-84.3 0.14 (1)	D-M	0 / 424	0.10 (1)	
D-E	-1498 / 0	-84.3	-84.3 0.32 (1)	M-E	-533 / 0	0.31 (1)	
E-F	-1498 / 0	-84.3	-84.3 0.32 (1)	M-F	0 / 424	0.10 (1)	
F-G	-1476 / 0	-84.3	-84.3 0.14 (1)	K-F	0 / 304	0.07 (2)	
G-H	0 / 17	-84.3	-84.3 0.15 (1)	K-G	0 / 119	0.03 (3)	
H-I	0 / 32	-84.3	-84.3 0.11 (1)	O-C	-1888 / 0	0.65 (1)	
O-B	-233 / 0	0.0	0.0 0.02 (1)	G-J	-1888 / 0	0.65 (1)	
J-H	-233 / 0	0.0	0.0 0.02 (1)				
O-N	0 / 1208	-28.0	-28.0 0.45 (2)				
N-M	0 / 1216	-28.0	-28.0 0.47 (2)				
M-L	0 / 1216	-28.0	-28.0 0.47 (2)				
L-K	0 / 1216	-28.0	-28.0 0.47 (2)				
K-J	0 / 1208	-28.0	-28.0 0.45 (2)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.82")
CALCULATED VERT. DEFL.(LL) = 1 / 999 (0.11")
ALLOWABLE DEFL.(TL) = L/360 (0.82")
CALCULATED VERT. DEFL.(TL) = 1 / 999 (0.19")

CSI: TC=0.32 (D-E:1), BC=0.47 (M-N:2), WB=0.65 (C-O:1), SS=0.21 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

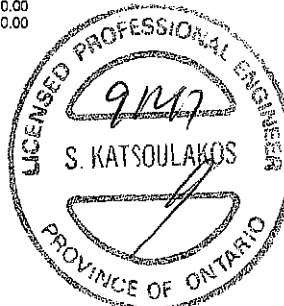
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1687 822 2284 1858

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (F) (INPUT = 0.80)
JSI METAL= 0.41 (C) (INPUT = 1.00)



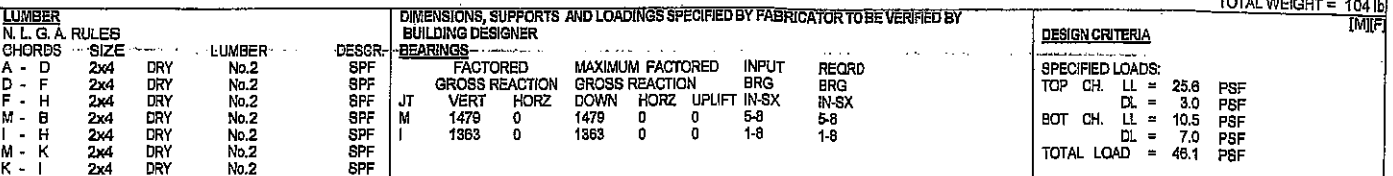
DRWG NO. TAM 46062.17
STRUCTURAL
COMPONENT ONLY



DWG NO. TAM 4009-18
STRUCTURAL
COMPONENT ONLY

N-M	0/1198	-28.0	-28.0	0.45 (2)	10.00
M-L	0/1201	-28.0	-28.0	0.46 (2)	10.00
L-K	0/1179	-28.0	-28.0	0.45 (2)	10.00
K-J	0/1179	-28.0	-28.0	0.45 (2)	10.00
J-I	0/1136	-28.0	-28.0	0.43 (2)	10.00

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.



UNFACTORED REACTIONS							SPACING = 24.0 IN. C/C	
1ST CASE		MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
M	1198	692/0	255/0	0/0	0/0	251/0	0/0	LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12
	1119	821/0	255/0	0/0	0/0	243/0	0/0	

PLATES (table ts in inches)						
PLATES	TYPE	PLATES	W	LEN	Y	X
A	TMV-p	MT20	3.0	4.0		
B	TMVW-t	MT20	4.0	6.0		
C	TMVW-t	MT20	4.0	4.0		
D	TMV-m	MT20	4.0	4.0		
E	TMVW-t	MT20	4.0	4.0		
F	TMV-m	MT20	4.0	4.0		
G	TMVW-t	MT20	4.0	6.0		
H	TMV-p	MT20	3.0	4.0		
I	BMVW-t	MT20	4.0	6.0		
J	BMVWWW-t	MT20	4.0	8.0		
K	BS-t	MT20	3.0	6.0		
L	BMVWWW-t	MT20	4.0	6.0		
M	BMVW-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS							
FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQ'D BRG		
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
M	1479	0	1479	0	0	5-8	5-8
I	1363	0	1363	0	0	1-8	1-8

UNFACTORED REACTIONS							
1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1198	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0
I	1119	821 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M.

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.32 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY
APPLIED

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO			FR-TO			
A-B	0/32	-84.3	-84.3	0.11 (1)	10.00	C-L	-117/85	0.07 (1)
B-C	0/22	-84.3	-84.3	0.24 (1)	10.00	L-D	0/495	0.11 (1)
C-D	-1389/0	-84.3	-84.3	0.21 (1)	5.32	L-E	-195/0	0.24 (1)
D-E	-1142/0	-84.3	-84.3	0.15 (1)	5.79	E-J	-225/0	0.27 (1)
E-F	-1128/0	-84.3	-84.3	0.15 (1)	5.82	J-F	0/484	0.11 (1)
F-G	-1371/0	-84.3	-84.3	0.20 (1)	5.35	J-G	-70/102	0.04 (1)
G-H	0/22	-84.3	-84.3	0.23 (1)	10.00	M-C	-1680/0	0.93 (1)
M-B	-255/0	0.0	0.0	0.03 (1)	7.81	G-I	-1632/0	0.86 (1)
I-H	-131/0	0.0	0.0	0.01 (1)	7.81			
M-L	0/1222	-28.0	-28.0	0.57 (2)	10.00			
L-K	0/1233	-28.0	-28.0	0.58 (2)	10.00			
K-J	0/1233	-28.0	-28.0	0.58 (2)	10.00			
J-I	0/1173	-28.0	-28.0	0.54 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL	=	25.6	PSF
		DL	=	3.0	PSF
BOT	CH.	LL	=	10.5	PSF
		DL	=	7.0	PSF
TOTAL LOAD				=	46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= $L/360$ (0.81")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.23")
ALLOWABLE DEFL.(TL)= $L/360$ (0.81")
CALCULATED VERT. DEFL.(TL) = $L/781$ (0.36")

CSI: TC=0.24/1.00 (B-C:1), BC=0.58/1.00 (J-L:2),
WB=0.93/1.00 (C-M:1), SS=0.19/1.00 (I-M:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

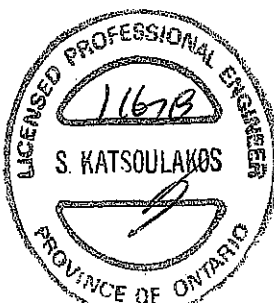
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)
	MAX	MIN	MAX	MIN	MAX MIN
MT20	618	354	1667	822	2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg

JSI GRIP= 0.89 (M) (INPUT = 0.90)
JSI METAL= 0.40 (C) (INPUT = 1.00)



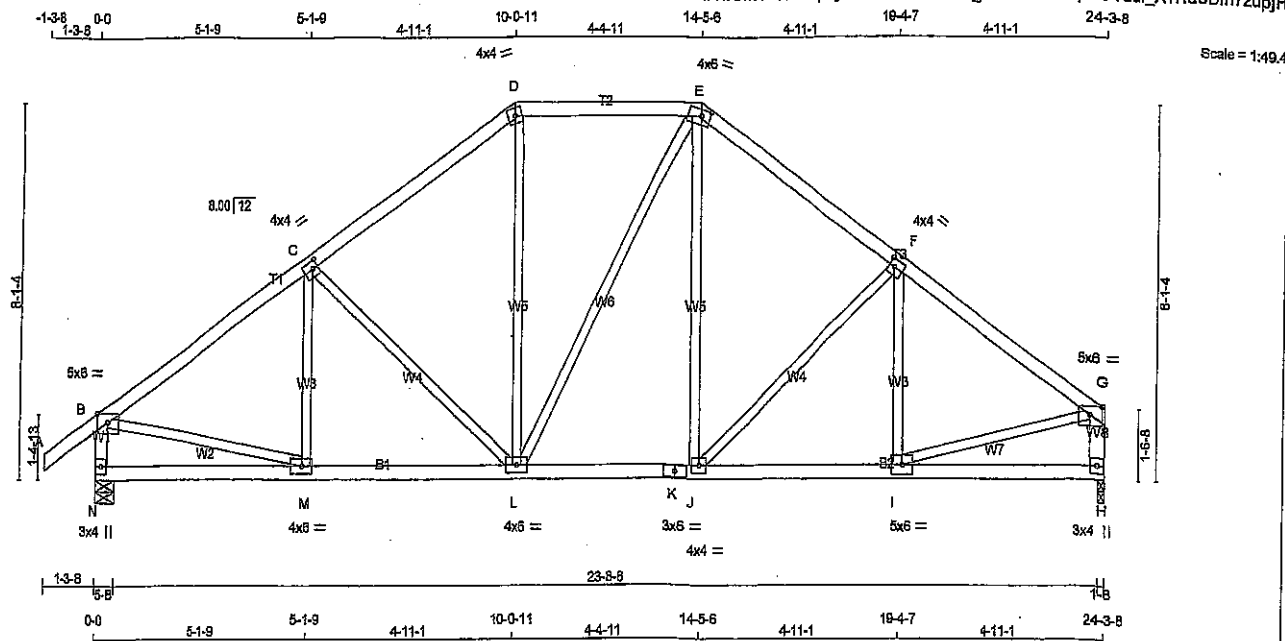
DWG NO. TAM 3985-18
STRUCTURAL
COMPONENT ONLY

JOB NAME 285994	TRUSS NAME T6A	QUANTITY 1	PLY 1	JOB DESC. 43954 TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 109 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
N - B	2x4	DRY	No.2	SPF	
H - G	2x4	DRY	No.2	SPF	
N - K	2x4	DRY	No.2	SPF	
K - H	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TTW-m	MT20	4.0	4.0		
E	TTW-m	MT20	4.0	6.0	1.75	2.50
F	TMVW-t	MT20	4.0	4.0	2.00	1.50
G	TMVW-p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMVW-t	MT20	5.0	6.0		
J	BMVW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMVW-t	MT20	4.0	6.0		
M	BMVW-t	MT20	4.0	6.0		
N	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	DOWN	GROSS REACTION	DOWN	BRG	IN-SX	BRG	IN-SX
N	1479	0	1479	0	0	5-8	5-8		
H	1363	0	1363	0	0	1-8	1-8		

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
N	1199	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0
H	1119	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)	UNBRACED LENGTH FR-TO
FR-TO				FR-TO			
A-B	0 / 32	-84.3	-84.3 0.11 (1)	10.00	M-C	-115 / 147	0.04 (1)
B-C	-1531 / 0	-84.3	-84.3 0.31 (1)	5.00	C-L	-354 / 0	0.30 (1)
C-D	-1271 / 0	-84.3	-84.3 0.29 (1)	5.39	L-D	0 / 374	0.08 (2)
D-E	-1037 / 0	-84.3	-84.3 0.22 (1)	5.91	L-E	0 / 15	0.00 (1)
E-F	-1282 / 0	-84.3	-84.3 0.28 (1)	5.42	J-E	0 / 360	0.08 (2)
F-G	-1493 / 0	-84.3	-84.3 0.29 (1)	5.08	J-F	-310 / 0	0.28 (1)
N-B	-1420 / 0	0.0	0.0 0.15 (1)	6.84	I-F	-155 / 122	0.05 (1)
H-G	-1307 / 0	0.0	0.0 0.14 (1)	7.08	B-M	0 / 1325	0.30 (1)
					I-G	0 / 1294	0.28 (1)
N-M	0 / 0	-28.0	-28.0 0.18 (3)	10.00			
M-L	0 / 1296	-28.0	-28.0 0.32 (2)	10.00			
L-K	0 / 1030	-28.0	-28.0 0.23 (2)	10.00			
K-J	0 / 1030	-28.0	-28.0 0.23 (2)	10.00			
J-I	0 / 1257	-28.0	-28.0 0.31 (2)	10.00			
I-H	0 / 0	-28.0	-28.0 0.17 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.8	PSF
	DL = 3.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 46.1	PSF

SPACING = 24.0 IN. O.C.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OSC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TRC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CS: TC=0.31/1.00 (B-C:1), BC=0.32/1.00 (L-M:2),
WB=0.30/1.00 (C-L:1), SS=0.17/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENB=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1657 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.89 (M) (INPUT = 0.80)
JSI METAL= 0.32 (M) (INPUT = 1.00)

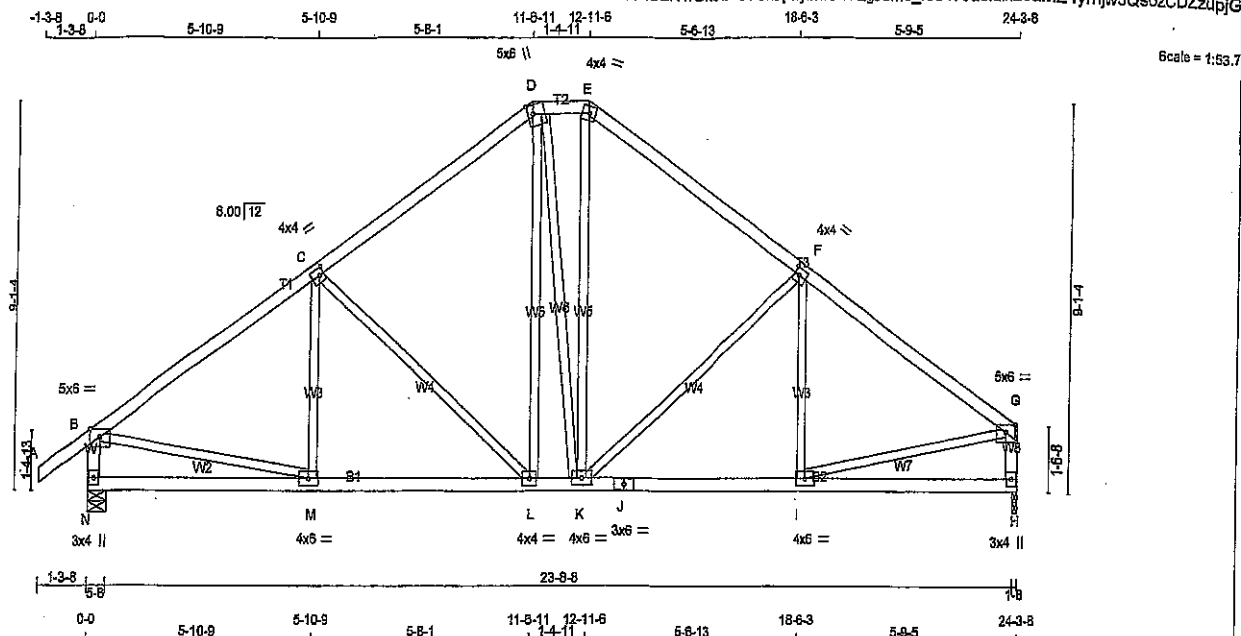


DRWG NO. TAM 3986-18
STRUCTURAL
COMPONENT ONLY

JOB NAME 285994	TRUSS NAME T7A	QUANTITY 1	PLY 1	JOB DESC. TRUSS DESC.	43954	DRWG NO.
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Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 115 lb

LUMBER			
N.L.G.A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
N - B	2x4	DRY	No.2
H - G	2x4	DRY	No.2
N - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
B	TMVW-p	MT20	5.0	6.0 Edge
C	TMVW-t	MT20	4.0	4.0 2.00 1.50
D	TTWV-m	MT20	5.0	6.0 2.50 1.50
E	TTW-m	MT20	4.0	4.0
F	TMVW-t	MT20	4.0	4.0 2.00 1.50
G	TMVW-p	MT20	5.0	6.0 Edge
H	BMV1-p	MT20	3.0	4.0
I	BMVW-t	MT20	4.0	6.0
J	BS-t	MT20	3.0	6.0
K	BMVW-t	MT20	4.0	6.0
L	BMVW-t	MT20	4.0	4.0
M	BMVW-t	MT20	4.0	6.0
N	BMV1-p	MT20	3.0	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	BRG	IN-SX
N	1479	0	1479	0	0	5-8	5-8	5-8	5-8
H	1363	0	1363	0	0	1-8	1-8	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
N	1198	682 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0
H	1119	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.68 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 32	-84.3 -84.3	0.11 (1)	10.00	M-C	-50 / 214	0.05 (3)
B-C	-1583 / 0	-84.3 -84.3	0.42 (1)	4.86	C-L	-481 / 0	0.57 (1)
C-D	-1183 / 0	-84.3 -84.3	0.38 (1)	5.44	L-D	0 / 377	0.08 (1)
D-E	-945 / 0	-84.3 -84.3	0.03 (1)	6.25	D-K	0 / 117	0.00 (2)
E-F	-1184 / 0	-84.3 -84.3	0.37 (1)	5.46	K-E	0 / 380	0.09 (1)
F-G	-1491 / 0	-84.3 -84.3	0.39 (1)	4.94	K-F	-433 / 0	0.51 (1)
N-B	-1413 / 0	0.0 0.0	0.15 (1)	6.86	I-F	-94 / 185	0.04 (3)
H-G	-1299 / 0	0.0 0.0	0.14 (1)	7.08	B-M	0 / 1324	0.30 (1)
					I-G	0 / 1296	0.29 (1)
N-M	0 / 0	-28.0 -28.0	0.25 (3)	10.00			
M-L	0 / 1302	-28.0 -28.0	0.40 (2)	10.00			
L-K	0 / 943	-28.0 -28.0	0.26 (2)	10.00			
K-J	0 / 1268	-28.0 -28.0	0.37 (2)	10.00			
J-I	0 / 1268	-28.0 -28.0	0.37 (2)	10.00			
I-H	0 / 0	-28.0 -28.0	0.23 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 085-09
- TPI/C 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL) = L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CS1: TC=0.42/1.00 (B-C:1), BC=0.40/1.00 (L-M:2), WB=0.57/1.00 (C-L:1), SS=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MT20	618	354	1687 622 2284 1658

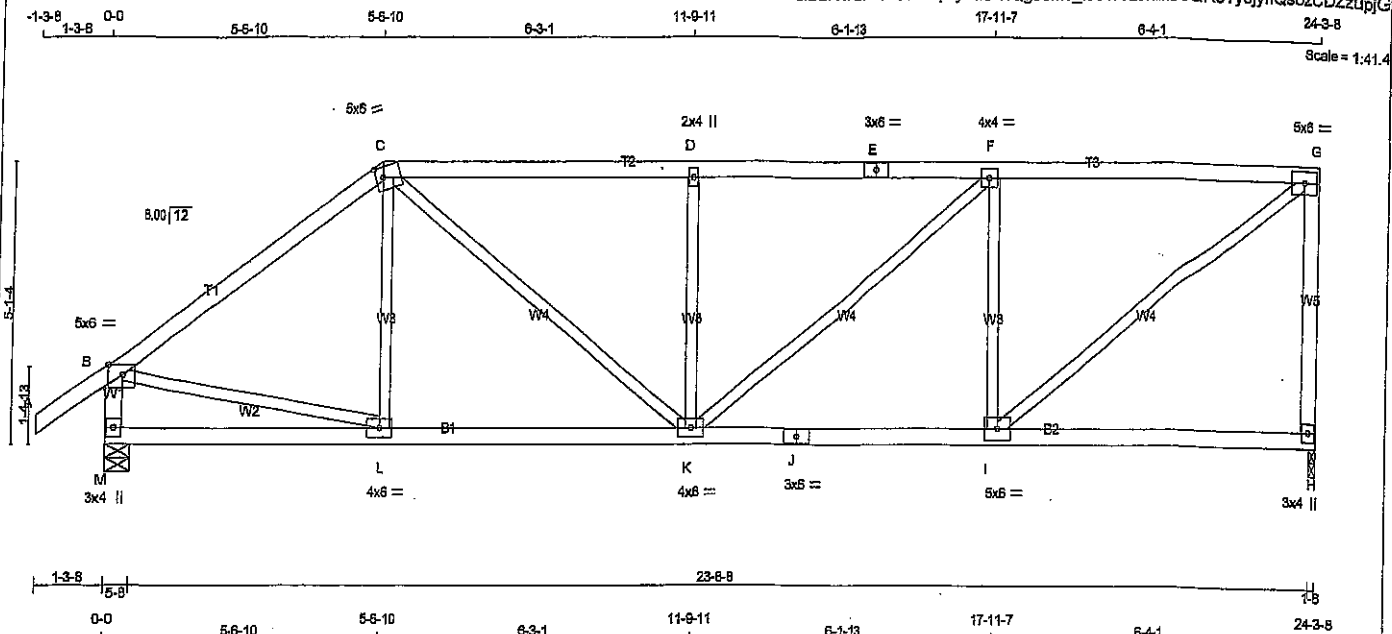
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.80)
JSI METAL= 0.33 (M) (INPUT = 1.00)



DRWG NO. TAM 3987-18
STRUCTURAL
COMPONENT ONLY



LUMBER
 N.L.G.A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
H - J	2x4	DRY	No.2
M - B	2x4	DRY	No.2
M - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
 EXCEPT
 DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMW-p	MT20	5.0	6.0	Edge	
C TTWW-m	MT20	5.0	6.0	2.25	1.75
D TMW-w	MT20	2.0	4.0		
E TS-t	MT20	3.0	6.0		
F TMWV-t	MT20	4.0	4.0		
G TMWV-t	MT20	5.0	6.0		
H BMV1+p	MT20	3.0	4.0		
I BMWV-t	MT20	5.0	6.0		
J BS-t	MT20	3.0	6.0		
K BMWV-t	MT20	4.0	6.0		
L BMWV-t	MT20	4.0	6.0		
M BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
 TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	VERT	HORZ	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
H	1363	0	1363	0	1-8
M	1479	0	1479	0	5-8

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
H	1119	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
M	1198	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.34 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)	MAX. UNBRACED LENGTH (FT)
FR-TO				FR-TO			
A-B	0 / 32	-84.3	-84.3 0.11 (1)	10.00	L-C	-49 / 204	0.05 (3)
B-C	-1493 / 0	-84.3	-84.3 0.55 (1)	4.89	B-L	0 / 1263	0.28 (1)
C-D	-1797 / 0	-84.3	-84.3 0.55 (1)	4.37	I-G	0 / 1778	0.40 (1)
D-E	-1797 / 0	-84.3	-84.3 0.56 (1)	4.34	C-K	0 / 702	0.16 (1)
E-F	-1797 / 0	-84.3	-84.3 0.56 (1)	4.34	I-F	-877 / 0	0.34 (1)
F-G	-1416 / 0	-84.3	-84.3 0.53 (1)	4.81	K-D	-566 / 0	0.22 (1)
H-G	-1292 / 0	0.0	0.0 0.57 (1)	7.10	K-F	0 / 484	0.11 (1)
M-B	-1417 / 0	0.0	0.0 0.15 (1)	6.65			
M-L	0 / 0	-28.0	-28.0 0.24 (3)	10.00			
L-K	0 / 1240	-28.0	-28.0 0.39 (2)	10.00			
K-J	0 / 1416	-28.0	-28.0 0.44 (2)	10.00			
J-I	0 / 1416	-28.0	-28.0 0.44 (2)	10.00			
I-H	0 / 0	-28.0	-28.0 0.27 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 25.6 PSF
 DL = 3.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A
 SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
 OR SMALL BUILDING REQUIREMENTS OF
 PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F.
 RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED
 ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360 (0.81")$
 CALCULATED VERT. DEFL.(LL) = $L/999 (0.09")$
 ALLOWABLE DEFL.(TL) = $L/360 (0.81")$
 CALCULATED VERT. DEFL.(TL) = $L/999 (0.14")$

CSI: TC=0.57/1.00 (G-H:1), BC=0.44/1.00 (H-K:2),
 WB=0.40/1.00 (G-I:1), SS=0.25/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT
 RESPONSIBLE FOR QUALITY CONTROL IN
 THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (K) (INPUT = 0.90)
 JSI METAL= 0.44 (J) (INPUT = 1.00)



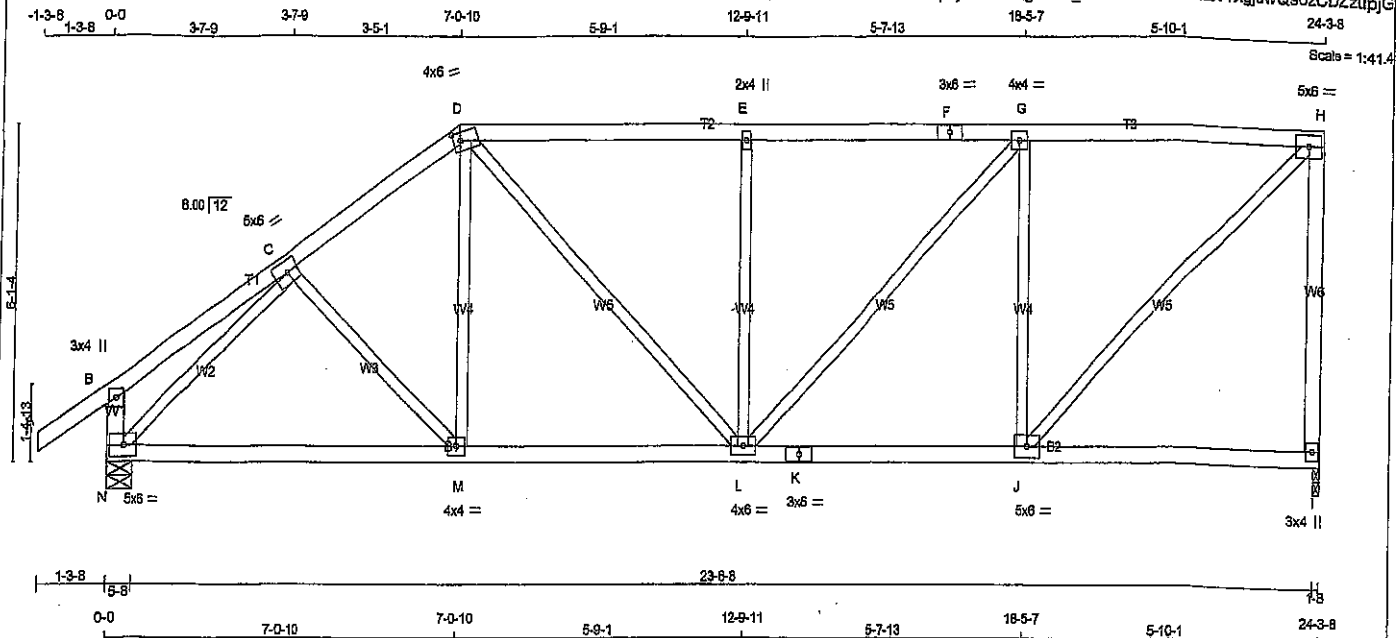
DRWG NO. TAM 3900-13
 STRUCTURAL
 COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
285994	T10	2	1	TRUSS DESC.		

Farmack Roof Truss, Burlington

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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - H	2x4	DRY	No.2	SPF	
I - H	2x4	DRY	No.2	SPF	
N - B	2x4	DRY	No.2	SPF	
N - K	2x4	DRY	No.2	SPF	
K - I	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-i	MT20	5.0	6.0		
D	TTVW-m	MT20	4.0	6.0	1.75	1.75
E	TMVW-w	MT20	2.0	4.0		
F	TS-i	MT20	3.0	6.0		
G	TMVW-i	MT20	4.0	4.0		
H	TMVW-i	MT20	5.0	6.0		
I	BMV1+p	MT20	3.0	4.0		
J	BMVW-i	MT20	5.0	6.0		
K	BS-i	MT20	3.0	6.0		
L	BMVW-i	MT20	4.0	6.0		
M	BMVW-i	MT20	4.0	4.0		
N	BMVW-i	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		GROSS REACTION		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX	IN-SX	IN-SX
I	1383	0	1353	0	0	1-8	1-8	1-8	1-8	1-8	1-8
N	1479	0	1479	0	0	5-8	5-8	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE	MAX./MIN.	COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1119	621/0	255/0	0/0	0/0	243/0	0/0	0/0	0/0
N	1198	682/0	255/0	0/0	0/0	251/0	0/0	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.86 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX. FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT.	LOAD LC1 (PLF)	MAX. CS1 (LC)	MAX. UNBRACED LENGTH	MEMB.	FORCE (LBS)	MAX. CS1 (LC)	MAX. UNBRACED LENGTH
FR-TO						FR-TO			
A-B	0/32	-84.3	-84.3	0.11 (1)	10.00	C-M	0/118	0.03 (3)	10.00
B-C	0/17	-84.3	-84.3	0.15 (1)	10.00	M-D	0/307	0.07 (2)	10.00
C-D	-1460/0	-84.3	-84.3	0.18 (1)	5.22	N-C	-1669/0	0.84 (1)	10.00
D-E	-1473/0	-84.3	-84.3	0.43 (1)	4.88	J-H	0/1557	0.35 (1)	10.00
E-F	-1473/0	-84.3	-84.3	0.44 (1)	4.88	D-L	0/385	0.09 (1)	10.00
F-G	-1473/0	-84.3	-84.3	0.44 (1)	4.88	J-G	-912/0	0.53 (1)	10.00
G-H	-1103/0	-84.3	-84.3	0.42 (1)	5.46	L-E	-520/0	0.30 (1)	10.00
I-H	-1298/0	0.0	0.0	0.94 (1)	7.09	L-G	0/530	0.12 (1)	10.00
N-B	-233/0	0.0	0.0	0.02 (1)	7.81				
N-M	0/1198	-28.0	-28.0	0.45 (2)	10.00				
M-L	0/1202	-28.0	-28.0	0.47 (2)	10.00				
L-K	0/1103	-28.0	-28.0	0.37 (2)	10.00				
K-J	0/1103	-28.0	-28.0	0.37 (2)	10.00				
J-I	0/0	-28.0	-28.0	0.24 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL) = L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/999 (0.19")

CS1: TC=0.94/1.00 (H-I:1), BC=0.47/1.00 (L-M:2),
WB=0.84/1.00 (C-N:1), SS=0.23/1.00 (G-H:1)

COL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

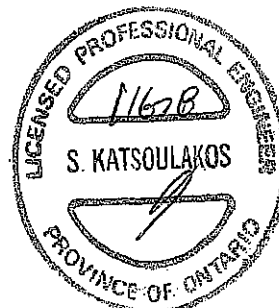
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1687 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.80 (D) (INPUT = 0.90)
JSI METAL= 0.41 (C) (INPUT = 1.00)

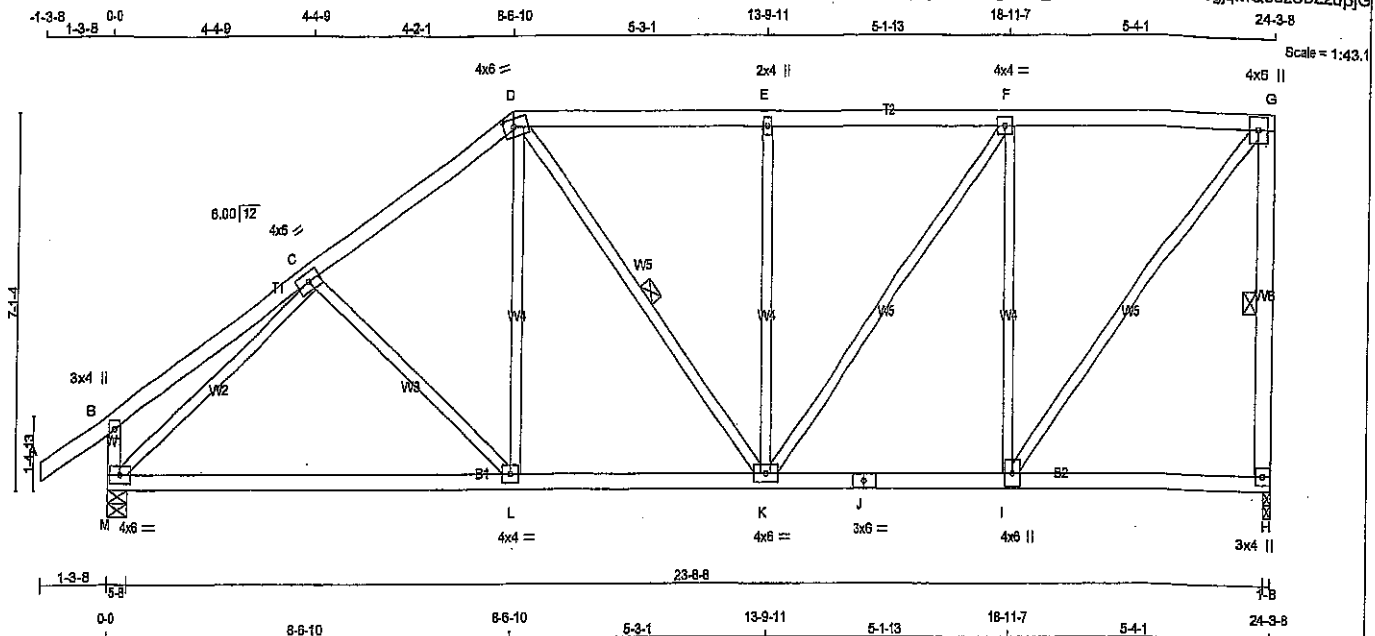


DRWG NO. TAM 3909 -18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
285994	T11	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:w3K2IB2K4fGk9fDO7SkJphylfMC-HQgJem0 J36475LefukB0aN21vgjqMQsozCDZupjG



TOTAL WEIGHT = 2 X 113 = 226 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - G	2x4	DRY	No.2	SPF	
H - G	2x4	DRY	No.2	SPF	
M - B	2x4	DRY	No.2	SPF	
M - J	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0			
C	TMVW+t	MT20	4.0	6.0			
D	TTWV+m	MT20	4.0	6.0	1.75	2.50	
E	TMVW+w	MT20	2.0	4.0			
F	TMVW+t	MT20	4.0	4.0			
G	TMVW+p	MT20	4.0	6.0			
H	BMV+t+p	MT20	3.0	4.0			
I	BMVW+t	MT20	4.0	6.0			
J	BS-t	MT20	3.0	6.0			
K	BMVWV+t	MT20	4.0	6.0			
L	BMVW+t	MT20	4.0	4.0			
M	BMVWV-t	MT20	4.0	6.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	VERT	GROSS REACTION	GROSS REACTION	DOWN	HORIZ	UPLIFT	IN-SX	IN-SX	IN-SX
H	1363	0	1393	0	0	1-5	1-5	1-5	1-5
M	1479	0	1479	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS		1ST LCASE		MAX/MIN. COMPONENT REACTIONS		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	JT	COMBINED	SNOW	LIVE
H	1119	621/0	255/0	0/0	0/0	243/0	0/0	H	1119	621/0	255/0
M	1199	692/0	255/0	0/0	0/0	251/0	0/0	M	1199	692/0	255/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-H, D-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO	LENGTH FR-TO
A-B	0/32	-84.3 -84.3 0.11 (1)	10.00
B-C	0/23	-84.3 -84.3 0.24 (1)	10.00
C-D	-1360/0	-84.3 -84.3 0.28 (1)	5.20
D-E	-1226/0	-84.3 -84.3 0.34 (1)	5.37
E-F	-1227/0	-84.3 -84.3 0.35 (1)	5.35
F-G	-879/0	-84.3 -84.3 0.34 (1)	6.07
H-G	-1306/0	0.0 0.0 0.30 (1)	5.66
M-B	-295/0	0.0 0.0 0.03 (1)	7.81
M-L	0/1223	-28.0 -28.0 0.59 (2)	10.00
L-K	0/1141	-28.0 -28.0 0.59 (2)	10.00
K-J	0/879	-28.0 -28.0 0.31 (2)	10.00
J-I	0/879	-28.0 -28.0 0.31 (2)	10.00
I-H	0/0	-28.0 -28.0 0.21 (3)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL)= L/899 (0.23")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL)= L/779 (0.37")

CSI: TC=0.35/1.00 (E-F:1), BC=0.59/1.00 (K-L:2), WB=0.93/1.00 (C-M:1), SS=0.21/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1667
	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (M) (INPUT = 0.90)
JSI METAL= 0.40 (C) (INPUT = 1.00)



DRWG NO. TAM 3990-18
STRUCTURAL
COMPONENT ONLY

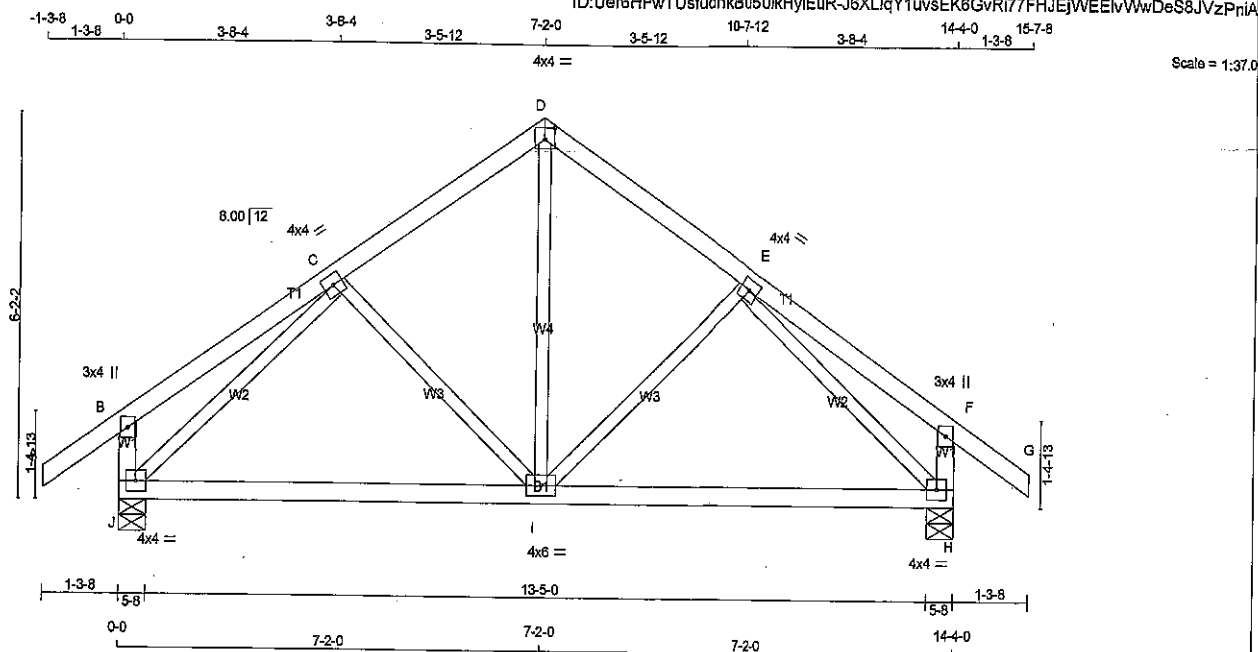
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JOB NAME 286071	TRUSS NAME T14	QUANTITY 4	PLY 1	JOB DESC. 43954	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:Uer6HFwTUsfudnkBu50kHylEuR-J6XLqY1uvsEK6GvR77FHJEJWEElnVwWDeS8JVzPnIA



TOTAL WEIGHT = 4 X 62 = 247 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - G	2x4	DRY	No.2	SPF	
J - B	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWV-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
J	920	0	920	0
H	920	0	920	0

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	739	437 / 0	151 / 0	0 / 0	0 / 0	152 / 0	0 / 0
H	739	437 / 0	151 / 0	0 / 0	0 / 0	152 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 32	-84.3	-84.3	0.11 (1)	I-D	0 / 480	0.10 (1)
B-C	0 / 19	-84.3	-84.3	0.17 (1)	I-E	-152 / 44	0.08 (1)
C-D	-545 / 0	-84.3	-84.3	0.13 (1)	C-I	-152 / 44	0.08 (1)
D-E	-545 / 0	-84.3	-84.3	0.13 (1)	J-C	-879 / 0	0.35 (1)
E-F	0 / 19	-84.3	-84.3	0.17 (1)	E-H	-879 / 0	0.35 (1)
F-G	0 / 32	-84.3	-84.3	0.11 (1)			
J-B	-232 / 0	0.0	0.0	0.02 (1)			
H-F	-232 / 0	0.0	0.0	0.02 (1)			
J-I	0 / 631	-28.0	-28.0	0.49 (2)			
I-H	0 / 631	-28.0	-28.0	0.49 (2)			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
DL = 3.0 PSF	
BOT CH.	LL = 10.5 PSF
DL = 7.0 PSF	
TOTAL LOAD	= 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.48")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.48")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.13")

CSI: TC=0.17/1.00 (E-F:1), BC=0.49/1.00 (H-I:2),
WB=0.35/1.00 (E-H:1), SSI=0.17/1.00 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	618	354	1867
	822	2284	1658

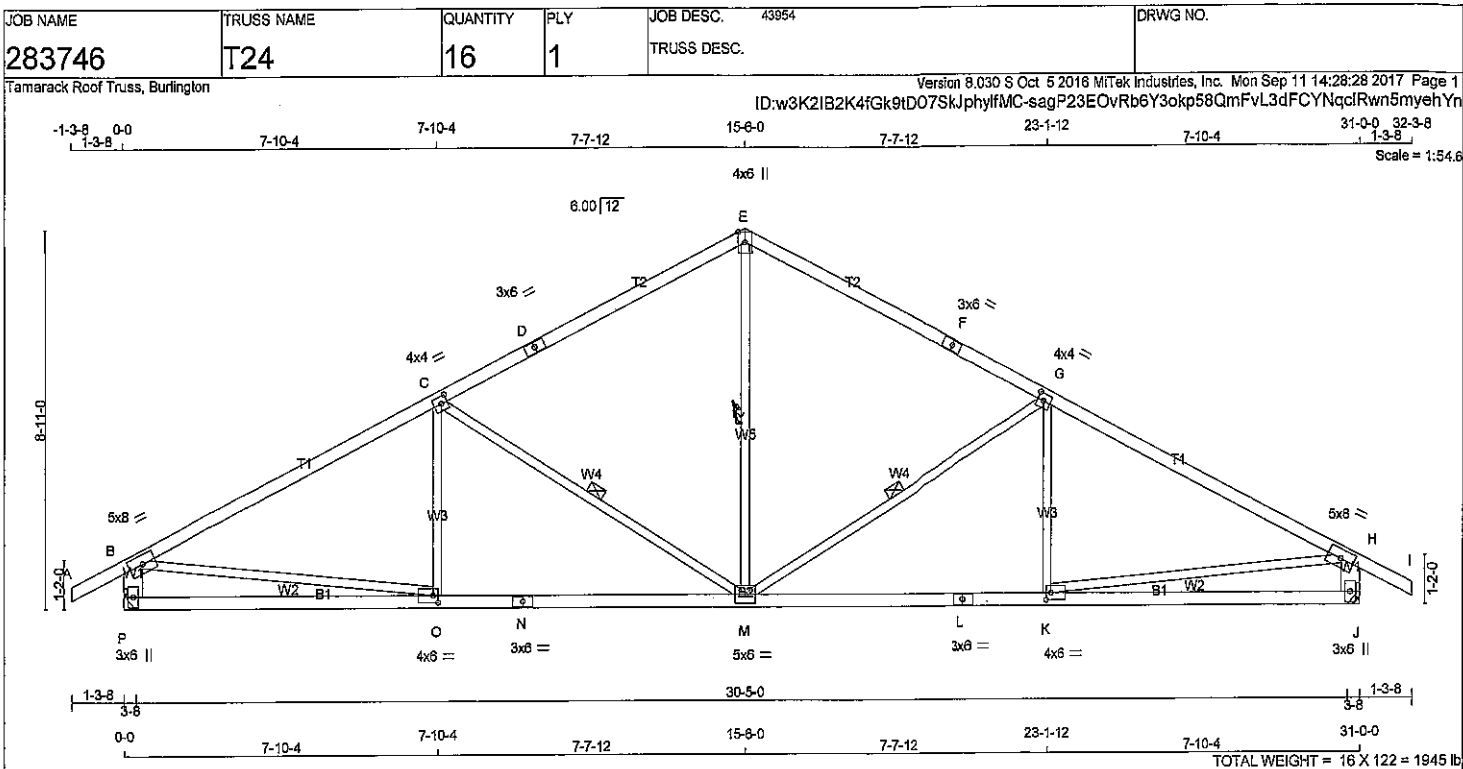
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (D) (INPUT = 0.90)
JSI METAL= 0.31 (C) (INPUT = 1.00)



DWG NO. TAM 21159-18
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
P - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	6.0		
E	TTW+p	MT20	4.0	8.0	Edge	
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
H	TMVW-t	MT20	5.0	8.0		
J	BMV1+p	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	6.0	2.00	1.50
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	5.0	6.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	6.0	2.00	1.50
P	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
P	1854	0	1854	0	HANGER BY OTHERS	HANGER BY OTHERS
J	1854	0	1854	0	HANGER BY OTHERS	HANGER BY OTHERS

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	WIND			
P	1506	883 / 0	328 / 0	0 / 0	0 / 0	318 / 0	0 / 0
J	1508	883 / 0	328 / 0	0 / 0	0 / 0	318 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.27 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-M, C-M. *E-M*

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LOAD LC1	MAX. CS1 (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CS1 (LC)	
FR-TO		FROM TO				FR-TO			
A-B	0 / 26	-84.3	-84.3	0.11 (1)	10.00	M-E	0 / 1098	0.25 (1)	
B-C	-2529 / 0	-84.3	-84.3	0.94 (1)	3.27	M-G	-840 / 0	0.48 (1)	
C-D	-1803 / 0	-84.3	-84.3	0.78 (1)	3.96	K-G	-9 / 307	0.07 (3)	
D-E	-1803 / 0	-84.3	-84.3	0.78 (1)	3.96	C-M	-840 / 0	0.48 (1)	
E-F	-1803 / 0	-84.3	-84.3	0.78 (1)	3.96	O-C	-9 / 307	0.07 (3)	
F-G	-1803 / 0	-84.3	-84.3	0.78 (1)	3.96	B-O	0 / 2308	0.52 (1)	
G-H	-2529 / 0	-84.3	-84.3	0.94 (1)	3.27	K-H	0 / 2308	0.52 (1)	
H-I	0 / 26	-84.3	-84.3	0.11 (1)	10.00				
P-B	-1764 / 0	0.0	0.0	0.11 (1)	7.48				
J-H	-1764 / 0	0.0	0.0	0.11 (1)	7.48				
P-O	0 / 0	-28.0	-28.0	0.42 (3)	10.00				
O-N	0 / 2293	-28.0	-28.0	0.68 (2)	10.00				
N-M	0 / 2293	-28.0	-28.0	0.68 (2)	10.00				
M-L	0 / 2293	-28.0	-28.0	0.68 (2)	10.00				
L-K	0 / 2293	-28.0	-28.0	0.68 (2)	10.00				
K-J	0 / 0	-28.0	-28.0	0.42 (3)	10.00				

USED PROFESSIONAL

9/2/11

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 3.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/999 (0.18")
ALLOWABLE DEFL.(TL) = L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/999 (0.30")

CSI: TC=0.94 (G-H:1), BC=0.68 (K-M:2),
WB=0.52 (H-K:1), SSI=0.29 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

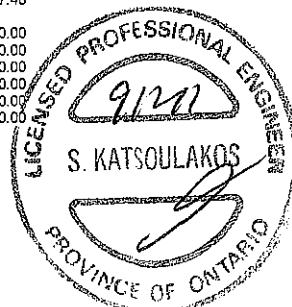
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	822	2284	1656

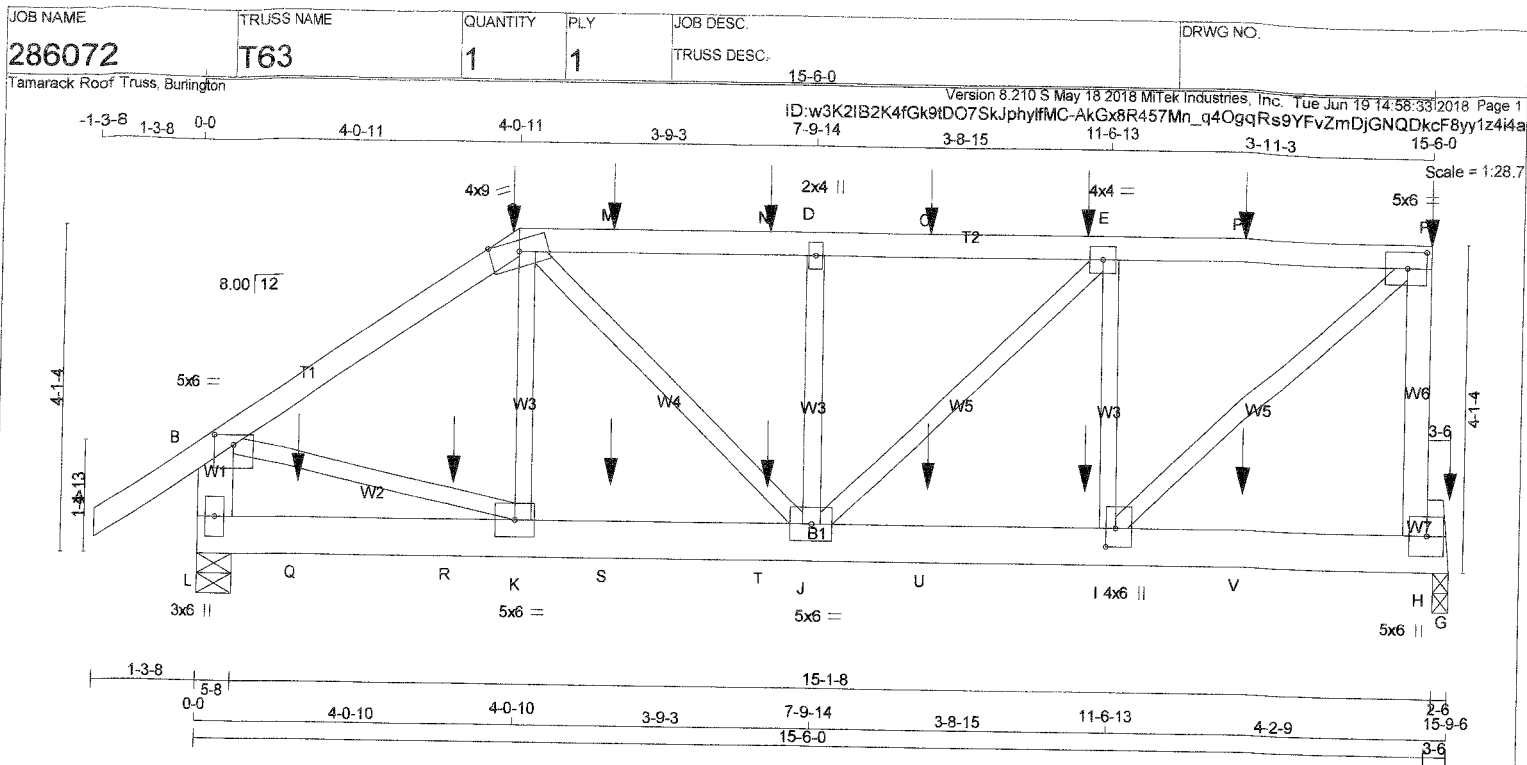
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (K) (INPUT = 0.90)
JSI METAL= 0.60 (N) (INPUT = 1.00)



DWG NO. TAN 4602017
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 78 lb									
[M]									
LUMBER									
N. L. G. A. RULES									
CHORDS SIZE									
LUMBER									
DESCR									
SPF									
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JOB NAME 286072	TRUSS NAME T63	QUANTITY 1	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

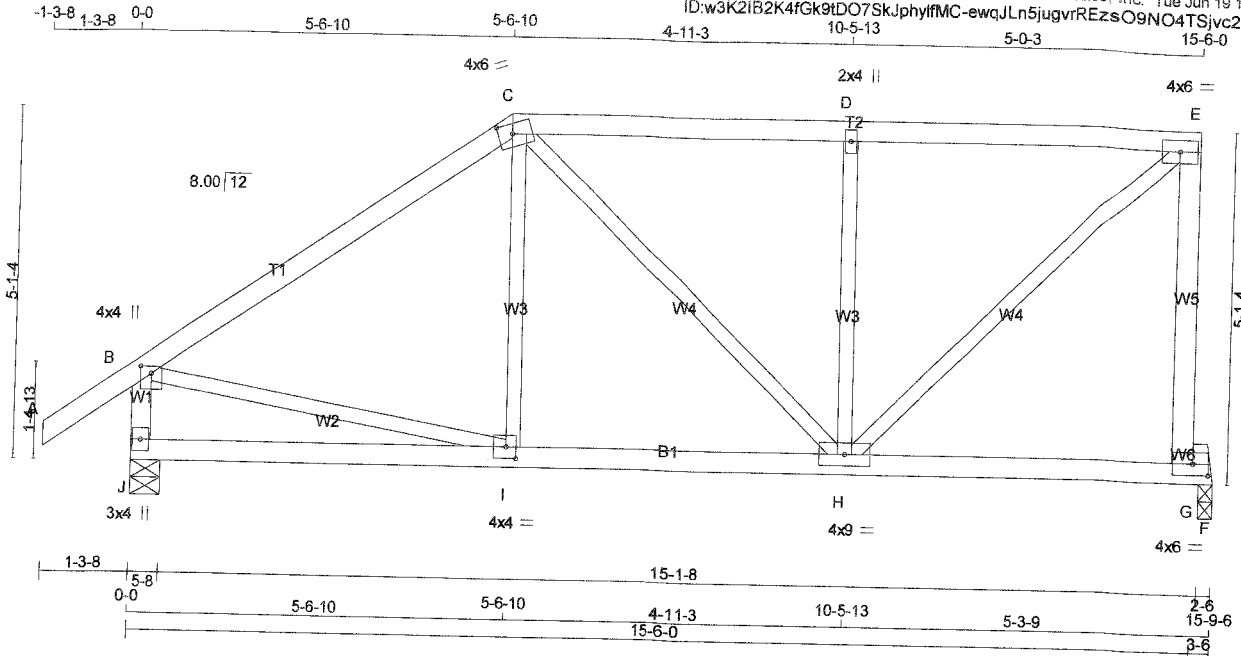
Version 8.210 S May 18 2018 MiTek Industries, Inc. Tue Jun 19 14:58:33 2018 Page 2
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SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
U	9-3-4	-50	-50	--	FRONT	VERT	TOTAL
V	13-3-4	-50	-50	--	FRONT	VERT	TOTAL



T1000542-2



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
G - E	2x4	DRY	No.2
J - B	2x4	DRY	No.2
J - F	2x4	DRY	No.2
G - F	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW+p	MT20	4.0	4.0	1.25	2.00
C	TTWW-m	MT20	4.0	6.0	1.75	2.50
D	TMW+w	MT20	2.0	4.0		
E	TMW-t	MT20	4.0	6.0		
G	BMVW-t	MT20	4.0	6.0	2.00	2.50
H	BMVW-w	MT20	4.0	9.0		
I	BMVW-t	MT20	4.0	4.0	2.00	1.75
J	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
J	1001	0	1001	0	5-8	5-8
F	862	0	862	0	2-6	2-6

UNFACTORED REACTIONS							
JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	806	474 / 0	196 / 0	0 / 0	0 / 0	166 / 0	0 / 0
F	711	389 / 0	166 / 0	0 / 0	0 / 0	156 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, F

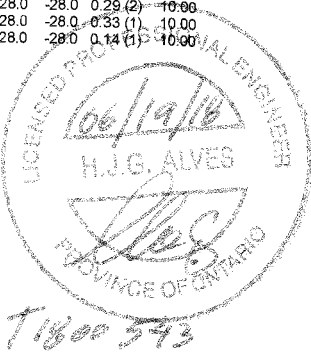
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.97 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED			MEMB.	WEBS MAX. FACTORED FORCE (LBS)
		VERT.	LOAD LC1	MAX. CS1 (LC)		
FR-TO					FR-TO	
A-B	0 / 32	-84.3	-84.3	0.11 (1)	I-C	0 / 210
B-C	-816 / 0	-84.3	-84.3	0.49 (1)	C-H	-8 / 62
C-D	-723 / 0	-84.3	-84.3	0.36 (1)	H-D	-517 / 0
D-E	-723 / 0	-84.3	-84.3	0.36 (1)	E-H	0 / 1003
G-E	-856 / 0	0.0	0.0	0.38 (1)	B-I	0 / 691
J-B	-933 / 0	0.0	0.0	0.10 (1)		
J-I	0 / 0	-28.0	-28.0	0.19 (2)		
I-H	0 / 679	-28.0	-28.0	0.29 (2)		
H-G	0 / 0	-28.0	-28.0	0.33 (1)		
G-F	0 / 0	-28.0	-28.0	0.14 (1)		



TOTAL WEIGHT = 68 lb (M)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.53")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (0.53")
CALCULATED VERT. DEFL.(TL) = L/999 (0.13")

CSI: TC=0.49/1.00 (B-C:1), BC=0.33/1.00 (G-H:1), WB=0.23/1.00 (E-H:1), SSI=0.34/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

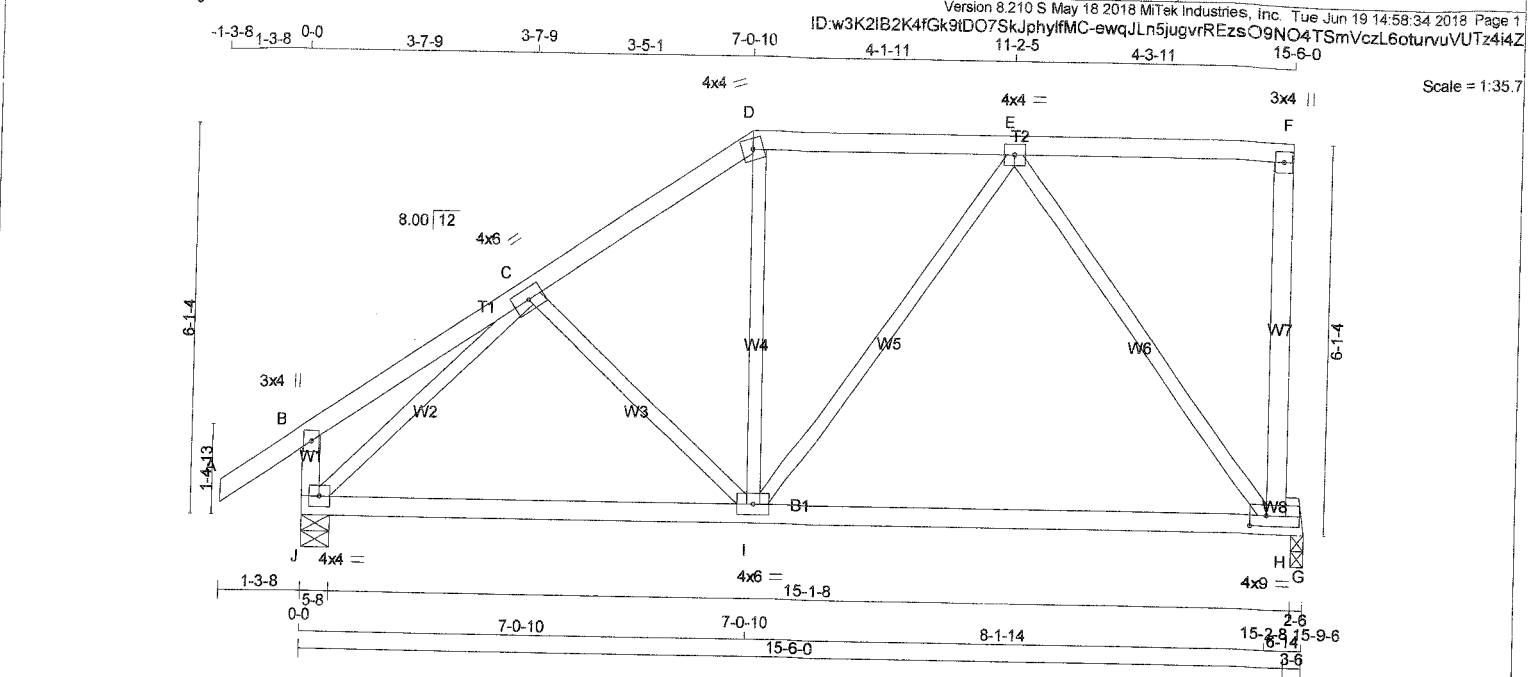
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (G) (INPUT = 0.90)
JSI METAL= 0.25 (G) (INPUT = 1.00)



LUMBER

N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVWW-t	MT20	4.0	9.0	2.00	3.00
I	BMVWW-t	MT20	4.0	6.0		
J	BMVW-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
G	862	0	862	0	2-6	2-6
J	1001	0	1001	0	5-8	5-8

UNFACTORED REACTIONS							
1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	711	389 / 0	166 / 0	0 / 0	0 / 0	156 / 0	0 / 0
J	806	474 / 0	166 / 0	0 / 0	0 / 0	166 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)		
FR-TO		FROM TO			FR-TO				
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	C-I	-122 / 58	0.05 (1)	
B-C	0 / 18	-84.3	-84.3	0.16 (1)	10.00	I-D	0 / 238	0.05 (2)	
C-D	-787 / 0	-84.3	-84.3	0.17 (1)	6.25	J-C	-1015 / 0	0.39 (1)	
D-E	-642 / 0	-84.3	-84.3	0.26 (1)	6.25	I-E	0 / 323	0.07 (2)	
E-F	0 / 0	-84.3	-84.3	0.25 (1)	10.00	E-H	-821 / 0	0.81 (1)	
H-F	-139 / 0	0.0	0.0	0.10 (1)	7.81				
J-B	-231 / 0	0.0	0.0	0.02 (1)	7.81				
J-I	0 / 728	-28.0	-28.0	0.65 (2)	10.00				
I-H	0 / 489	-28.0	-28.0	0.62 (2)	10.00				
H-G	0 / 0	-28.0	-28.0	0.14 (1)	10.00				

TOTAL WEIGHT = 70 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:	
TOP CH. LL	= 25.6 PSF
DL	= 3.0 PSF
BOT CH. LL	= 10.5 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.53")
CALCULATED VERT. DEFL. (LL) = L/713 (0.27")
ALLOWABLE DEFL. (TL) = L/360 (0.53")
CALCULATED VERT. DEFL. (TL) = L/433 (0.44")

CSI: TC=0.26/1.00 (D-E:1), BC=0.65/1.00 (I-J:2), WB=0.81/1.00 (E-H:1), SS=0.34/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

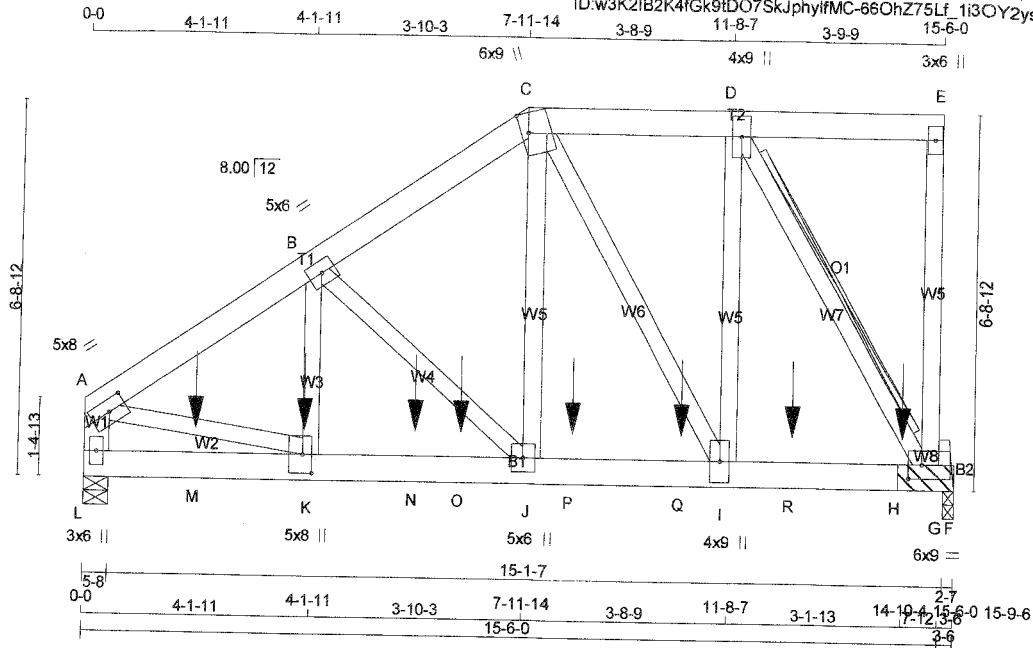
JSI GRIP= 0.85 (J) (INPUT = 0.90)
JSI METAL= 0.27 (J) (INPUT = 1.00)



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286072	T66Z8	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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Scale = 1:41.1

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x6 DRY No.2	SPF			
C - E	2x6 DRY No.2	SPF			
G - E	2x4 DRY No.2	SPF			
L - A	2x6 DRY No.2	SPF			
L - F	2x6 DRY 2100F 1.8E	SPF			
G - F	2x6 DRY No.2	SPF			

ALL WEBS 2x4 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C 2	12	TOP
C-E 2	12	TOP
L-A 2	12	TOP
E-G 1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
L-F 2	6	SIDE (1094.5)
WEBS : (0.122"x3") SPIRAL NAILS		
2x4 1	6	
2x6 2	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMVW-t MT20	5.0	8.0	2.50	3.75
B	TMVW-t MT20	5.0	6.0		
C	TTVW+m MT20	6.0	9.0	4.25	1.50
D	TMVW-t MT20	4.0	9.0		
E	TMV+p MT20	3.0	6.0		
G	BMVWVW-t MT20	6.0	9.0	3.00	3.00
I	BMVW-t MT20	4.0	9.0		
J	BMVW-t MT20	5.0	6.0		
K	BMVW-t MT20	5.0	8.0	4.00	2.00
L	BMV1+p MT20	3.0	6.0		

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	IN-SX	IN-SX
L	7635 0	7635 0	5-8	5-8
F	8152 0	8152 0	2-7	2-7 & BLOCK

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. COMPONENT REACTIONS	LIVE	PERM. LIVE	WIND	DEAD	SOIL
L	6187	3567 / 0	1321 / 0	0 / 0	0 / 0	1299 / 0	0 / 0
F	6634	3779 / 0	1447 / 0	0 / 0	0 / 0	1408 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, F

2x6 DRY SPF 2100F 1.8E BEARING BLOCK 12" LONG AT JT, F ATTACHED TO FRONT SIDE WITH 3 ROWS OF (0.122"x3") SPIRAL NAILS SPACED 3" C.C. 12 NAILS TOTAL.

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.77 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x6 DRY SPF No.2 T-BRACE AT D-G

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH
FR-TO					FR-TO			
A-B	-9171 / 0	-84.3	-84.3 0.10 (1)	4.77	K-B	0 / 3006	0.16 (1)	
B-C	-6838 / 0	-84.3	-84.3 0.07 (1)	5.43	B-J	-2834 / 0	0.31 (1)	
C-D	-4312 / 0	-84.3	-84.3 0.03 (1)	6.25	J-C	0 / 5626	0.30 (1)	
D-E	0 / 0	-84.3	-84.3 0.02 (1)	10.00	C-I	-2631 / 0	0.58 (1)	
E-F	-133 / 0	0.0	0.0 0.04 (1)	7.81	I-D	0 / 6780	0.36 (1)	
L-A	-6933 / 0	0.0	0.0 0.15 (1)	6.75	D-G	-6328 / 0	0.53 (1)	
L-M	0 / 0	-28.0	-28.0 0.17 (1)	10.00	A-K	0 / 7791	0.42 (1)	
M-K	0 / 0	-28.0	-28.0 0.17 (1)	10.00				
K-N	0 / 7615	-28.0	-28.0 0.33 (1)	10.00				
N-O	0 / 7615	-28.0	-28.0 0.33 (1)	10.00				
O-J	0 / 7615	-28.0	-28.0 0.33 (1)	10.00				
J-P	0 / 5656	-28.0	-28.0 0.31 (1)	10.00				
P-Q	0 / 5656	-28.0	-28.0 0.31 (1)	10.00				
Q-I	0 / 5656	-28.0	-28.0 0.31 (1)	10.00				
I-R	0 / 4312	-28.0	-28.0 0.42 (1)	10.00				
R-H	0 / 4312	-28.0	-28.0 0.42 (1)	10.00				
H-G	0 / 4312	-28.0	-28.0 0.42 (1)	10.00				
G-F	0 / 0	-28.0	-28.0 0.21 (1)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
H	14-10-4	-1395	-1395	---	BACK	VERT	TOTAL
K	4-0-4	-1471	-1471	---	BACK	VERT	TOTAL
M	2-0-4	-1471	-1471	---	BACK	VERT	TOTAL
N	6-0-4	-1471	-1471	---	BACK	VERT	TOTAL
O	6-10-4	-1394	-1394	---	BACK	VERT	TOTAL
P	8-10-4	-1394	-1394	---	BACK	VERT	TOTAL

TOTAL WEIGHT = 3 X 112 = 335 lb

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.53")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (0.53")
CALCULATED VERT. DEFL.(TL) = L/999 (0.12")

CSI: TC=0.15/1.00 (A-L:1), BC=0.42/1.00 (G-I:1), WB=0.58/1.00 (C-I:1), SSI=0.82/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (G) (INPUT = 0.90)

JSI METAL= 0.66 (K) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME 286072	TRUSS NAME T66Z8	QUANTITY 1	PLY 3	JOB DESC. TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1826.2 lbs FACTORED DOWN AT 2-0-4, 1826.2 lbs FACTORED DOWN AT 4-0-4, 1826.2 lbs FACTORED DOWN AT 6-0-4, 1711.9 lbs FACTORED DOWN AT 6-10-4, 1711.9 lbs FACTORED DOWN AT 8-10-4, 1711.9 lbs FACTORED DOWN AT 10-10-4, AND 1711.9 lbs FACTORED DOWN AT 12-10-4, AND 1712.8 lbs FACTORED DOWN AT 14-10-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
Q	10-10-4	-1394	-1394	---	BACK	VERT	TOTAL
R	12-10-4	-1394	-1394	---	BACK	VERT	TOTAL



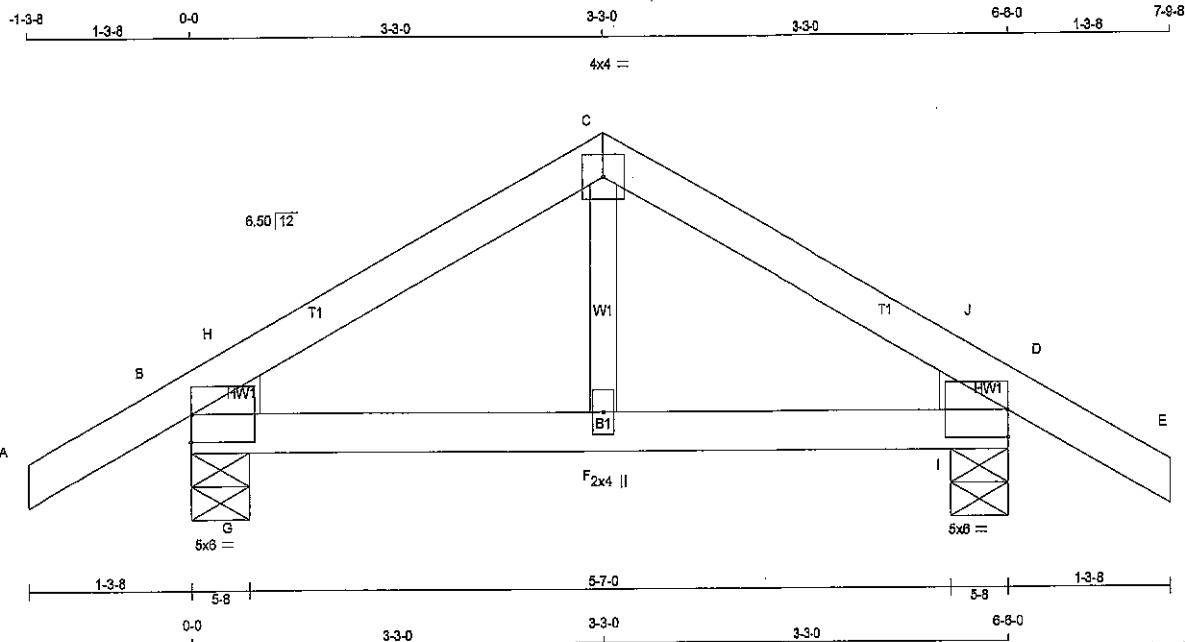
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
283746	T93	4	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1:17.3

TOTAL WEIGHT = 4 X 22 = 88 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
B - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
		DRY: SEASONED LUMBER.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-I	MT20	5.0	6.0		
C	TTW-p	MT20	4.0	4.0		
D	TMBH1-I	MT20	5.0	6.0		
F	BMW+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG	HEEL
JT	VERT	HORZ	DOWN	HORZ	WEDGE
B	480	0	480	0	0
D	480	0	480	0	0

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	377	236 / 0	66 / 0	0 / 0	0 / 0	73 / 0	0 / 0
D	377	236 / 0	66 / 0	0 / 0	0 / 0	73 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED HORIZ. LOAD (PLF)	MAX. MEMB. LENGTH (FT)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (PLF)
FR-TO							
A-B	0 / 17	-84.3	-84.3	0.11 (1)	10.00	F-C	0 / 165
B-H	-412 / 0	-84.3	-84.3	0.05 (1)	6.25	G-H	-34 / 113
H-C	-355 / 0	-84.3	-84.3	0.08 (1)	6.25	I-J	-34 / 113
C-J	-355 / 0	-84.3	-84.3	0.08 (1)	6.25		
J-D	-412 / 0	-84.3	-84.3	0.05 (1)	6.25		
D-E	0 / 17	-84.3	-84.3	0.11 (1)	10.00		
B-G	0 / 305	-28.0	-28.0	0.08 (1)	10.00		
G-F	0 / 305	-28.0	-28.0	0.10 (1)	10.00		
F-I	0 / 305	-28.0	-28.0	0.10 (1)	10.00		
I-D	0 / 305	-28.0	-28.0	0.08 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= 1/360 (0.22")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.00")
ALLOWABLE DEFL.(TL)= 1/360 (0.22")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.01")

CSI: TC=0.11 (D-E:1), BC=0.10 (F-G:1), WB=0.04 (C-F:2), SS=0.09 (C-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	518	354	1687
	822	2284	1656

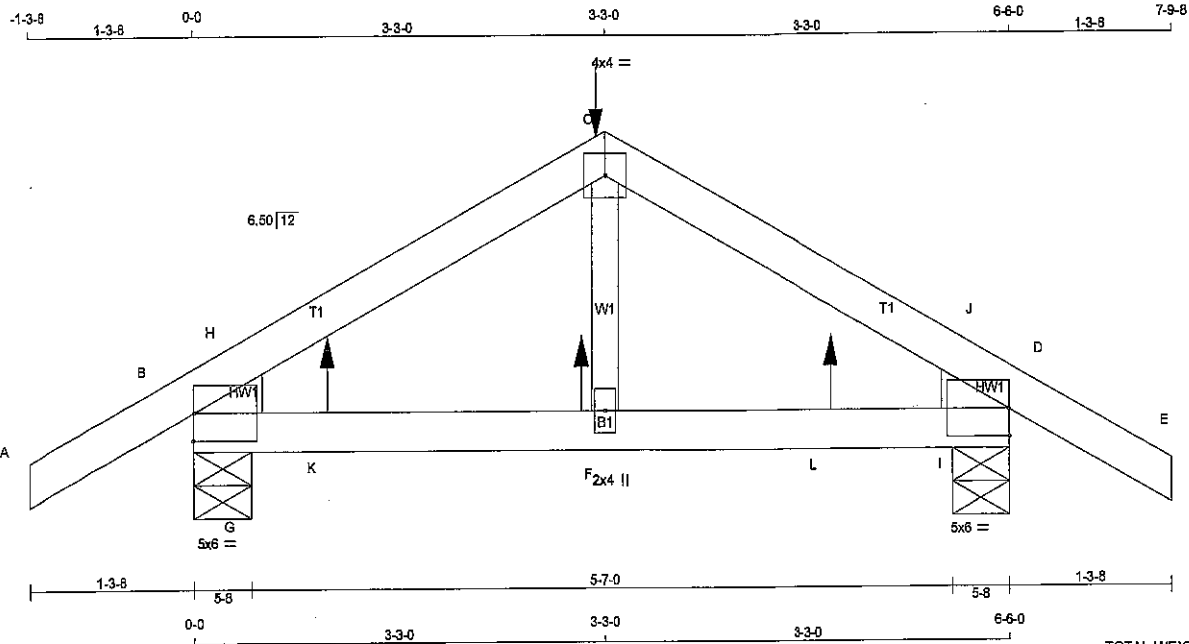
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (D) (INPUT = 0.80)
JSI METAL= 0.10 (B) (INPUT = 1.00)



DRWG NO. TAM 46075-17
STRUCTURAL
COMPONENT ONLY



LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	SPF
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
B	TMBH1-I	MT20	5.0	6.0
C	TTW-p	MT20	4.0	4.0
D	TMBH1-I	MT20	5.0	6.0
F	BMW+ww	MT20	2.0	4.0

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1.7 lbs FACTORED DOWN AND 105.9 lbs FACTORED UP AT 3-3-0 ON TOP CHORD, AND 16.8 lbs FACTORED UP AT 1-0-12, AND 16.8 lbs FACTORED UP AT 3-0-12, AND 16.8 lbs FACTORED UP AT 5-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD	HEEL
JT	GROSS REACTION	GROSS REACTION	GROSS REACTION	BRG	IN-SX	WEDGE
B	462	0	462	0	5-8	2x4 L
D	464	0	464	0	5-8	2x4 R

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN COMPONENT REACTIONS	1ST LCASE	MAX/MIN COMPONENT REACTIONS
B	COMBINED	SNOW	LIVE	PERM.LIVE
B	366	224 / 0	69 / 0	0 / 0
D	367	225 / 0	69 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS					WEBS				
MAX. FACTORED		FACTORED			MAX. FACTORED		FACTORED		
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX. CS1 (LC)	MAX. UNBRAC	MEMB.	FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM	TO		LENGTH	FR-TO			
A-B	0 / 17	-84.3	-84.3	0.12 (1)	10.00	F-C	0 / 162	0.04 (7)	
B-H	-383 / 0	-84.3	-84.3	0.07 (1)	6.25	G-H	-94 / 113	0.00 (1)	
H-C	-336 / 0	-84.3	-84.3	0.10 (5)	6.25	I-J	-95 / 113	0.00 (1)	
C-J	-336 / 0	-84.3	-84.3	0.10 (5)	6.25				
J-D	-383 / 0	-84.3	-84.3	0.07 (1)	6.25				
D-E	0 / 17	-84.3	-84.3	0.12 (1)	10.00				
B-G	0 / 289	-28.0	-28.0	0.09 (5)	10.00				
G-K	0 / 289	-28.0	-28.0	0.11 (1)	10.00				
K-F	0 / 289	-28.0	-28.0	0.11 (1)	10.00				
F-L	0 / 289	-28.0	-28.0	0.11 (1)	10.00				
L-I	0 / 289	-28.0	-28.0	0.11 (1)	10.00				
I-D	0 / 289	-28.0	-28.0	0.10 (5)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.
C	3-3-0	-1	-2	105	BACK	VERT
F	3-0-12	11	-	17	BACK	VERT
K	1-0-12	11	-	17	BACK	VERT
L	5-0-12	11	-	17	BACK	VERT

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN. G/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.22")
CALCULATED VERT. DEFL. (LL) = $L/899$ (0.00")
ALLOWABLE DEFL. (TL) = $L/360$ (0.22")
CALCULATED VERT. DEFL. (TL) = $L/899$ (0.01")

CS1: TC=0.12 (D-E:1), BC=0.11 (F-G:1), WB=0.04 (C-F:7), SS=0.10 (C-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(FSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1658

PLATE PLACEMENT TOL. = 0.250 inches

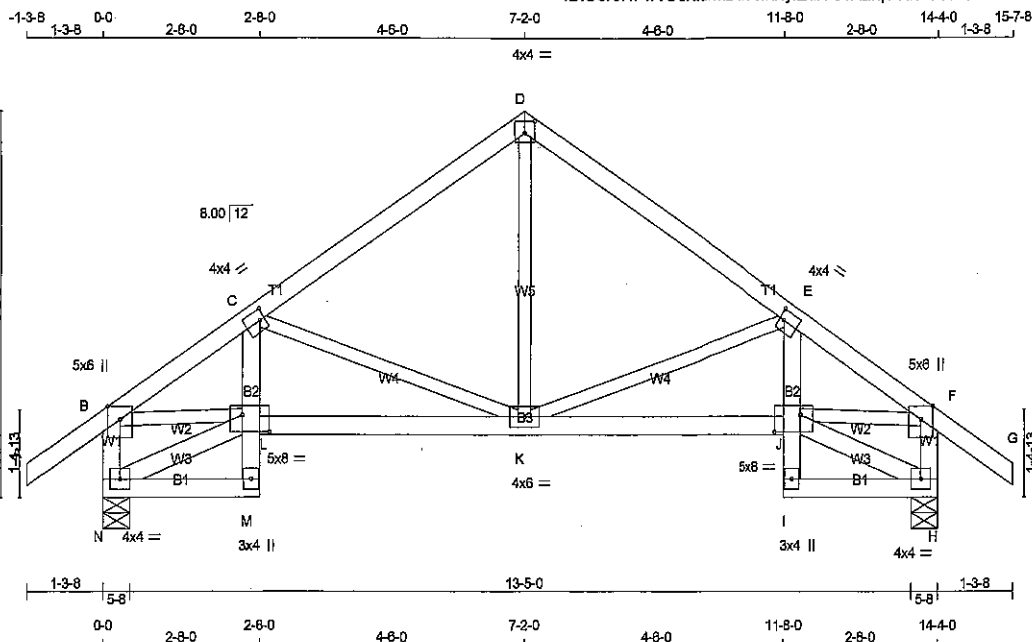
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (D) (INPUT = 0.80)
JSI METAL= 0.10 (B) (INPUT = 1.00)



DWG NO. TAM 46076-17
STRUCTURAL
COMPONENT ONLY

JOB NAME 286071	TRUSS NAME T112	QUANTITY 2	PLY 1	JOB DESC. 43954	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Apr 17 15:24:35 2018 Page 1	ID:Uer6HFwTUsfudnkBu50kHylEuR-J6XLIqY1uvsEK8GvRI77FHJDtWHDlxFwDeS8JVzPnIA



TOTAL WEIGHT = 2 X 88 = 136 lb

LUMBER				N. L. G. A. RULES	
CHORDS	SIZE	LUMBER	DESCR.	SPF	
A - D	2x4	DRY	No.2	SPF	
D - G	2x4	DRY	No.2	SPF	
N - B	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
N - M	2x4	DRY	No.2	SPF	
M - C	2x4	DRY	No.2	SPF	
L - J	2x4	DRY	No.2	SPF	
I - E	2x4	DRY	No.2	SPF	
I - H	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
N - L	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMVW-t	MT20	4.0	4.0	2.00	1.00
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMVW-t	MT20	4.0	4.0	2.00	1.00
F	TMVW+p	MT20	5.0	6.0	Edge	
H	BMVW1-t	MT20	4.0	4.0		
I	BMV+p	MT20	3.0	4.0		
J	BVMWW-t	MT20	5.0	8.0	3.25	5.50
K	BVMWW-t	MT20	4.0	6.0		
L	BVMWW-t	MT20	5.0	8.0	3.25	5.50
M	BMV+p	MT20	3.0	4.0		
N	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
N	920	0	920	0
H	920	0	920	0

UNFACTORED REACTIONS

JT	1ST CASE	MAX / MIN	COMPONENT REACTIONS	WIND	DEAD	SOIL
N	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
N	739	437 / 0	150 / 0	0 / 0	0 / 0	152 / 0
H	739	437 / 0	150 / 0	0 / 0	0 / 0	152 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.60 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)	
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	K-D	0 / 482
B-C	-1234 / 0	-84.3	-84.3	0.17 (1)	5.60	K-E	-500 / 0
C-D	-748 / 0	-84.3	-84.3	0.22 (1)	8.25	C-K	-500 / 0
D-E	-748 / 0	-84.3	-84.3	0.22 (1)	6.25	N-L	-40 / 0
E-F	-1234 / 0	-84.3	-84.3	0.17 (1)	5.60	B-L	0 / 1053
F-G	0 / 32	-84.3	-84.3	0.11 (1)	10.00	J-H	-40 / 0
N-B	-889 / 0	0.0	0.0	0.09 (1)	7.81	J-F	0 / 1053
H-F	-889 / 0	0.0	0.0	0.09 (1)	7.81		
N-M	0 / 37	-28.0	-28.0	0.06 (2)	10.00		
M-L	0 / 62	0.0	0.0	0.06 (1)	10.00		
L-C	0 / 147	0.0	0.0	0.07 (1)	10.00		
L-K	0 / 1076	-28.0	-28.0	0.30 (2)	10.00		
K-J	0 / 1076	-28.0	-28.0	0.30 (2)	10.00		
I-J	0 / 62	0.0	0.0	0.06 (1)	10.00		
J-E	0 / 147	0.0	0.0	0.07 (1)	10.00		
I-H	0 / 37	-28.0	-28.0	0.06 (2)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 3.0 PSF
BOT CH. LL	= 10.5 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA D68-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.48")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.48")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.22/1.00 (D-E:1), BC=0.30/1.00 (J-K:2), WB=0.24/1.00 (F-J:1), SS=0.15/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (L) (INPUT = 0.90)
JSI METAL= 0.24 (F) (INPUT = 1.00)



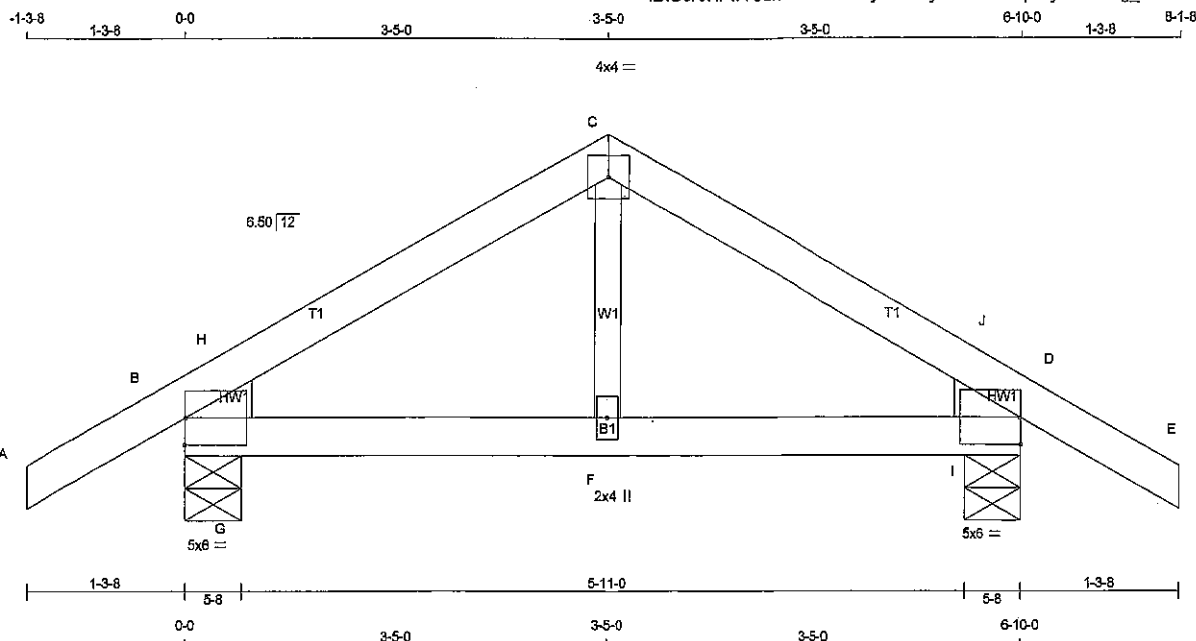
DRWG NO. TAM 21160-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
283746	T129	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:Uer6HFwTUsfudnkBu50kHylEuR-oyc9TIFeR2sqnMy7xWVAusg_uVR4U0PpuDIPtAfyehY



Scale = 1:17.5

LUMBER
N. L. G. A. RULES
CHORDS SIZE
A - C 2x4 DRY
C - E 2x4 DRY
B - D 2x4 DRY
ALL WEBS 2x3 DRY
DRY: SEASONED LUMBER.

LUMBER
DESCR.
SPF
SPF
SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX	HEEL WEDGE
	VERT	HORZ	DOWN	HORZ			
B	498	0	498	0	5-8	5-8	2x4 L
D	498	0	498	0	5-8	5-8	2x4 R

UNFACTORED REACTIONS

JT	COMBINED	MAX./MIN. COMPONENT REACTIONS				WIND	DEAD	SOIL
		1ST LCASE	SNOW	LIVE	PERM. LIVE			
B	393	244 / 0	72 / 0	0 / 0	0 / 0	77 / 0	0 / 0	0 / 0
D	393	244 / 0	72 / 0	0 / 0	0 / 0	77 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM	TO		LENGTH	FR-TO			
A-B	0 / 17	-84.3	-84.3	0.11 (1)	10.00	F-C	0 / 176	0.04 (2)	
B-H	-431 / 0	-84.3	-84.3	0.08 (1)	6.25	G-H	-42 / 121	0.00 (1)	
H-C	-377 / 0	-84.3	-84.3	0.09 (1)	6.25	I-J	-42 / 121	0.00 (1)	
C-J	-377 / 0	-84.3	-84.3	0.09 (1)	6.25				
J-D	-431 / 0	-84.3	-84.3	0.08 (1)	6.25				
D-E	0 / 17	-84.3	-84.3	0.11 (1)	10.00				
B-G	0 / 324	-28.0	-28.0	0.09 (1)	10.00				
G-F	0 / 324	-28.0	-28.0	0.11 (1)	10.00				
F-I	0 / 324	-28.0	-28.0	0.11 (1)	10.00				
I-D	0 / 324	-28.0	-28.0	0.09 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.23")
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.23")
CALCULATED VERT. DEFL.(TL)= L/999 (0.01")

CSI: TC=0.11 (D-E:1), BC=0.11 (F-G:1), WB=0.04 (C-F:2), SSI=0.10 (C-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1658

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.82 (D) (INPUT = 0.90)
JSI METAL= 0.11 (D) (INPUT = 1.00)

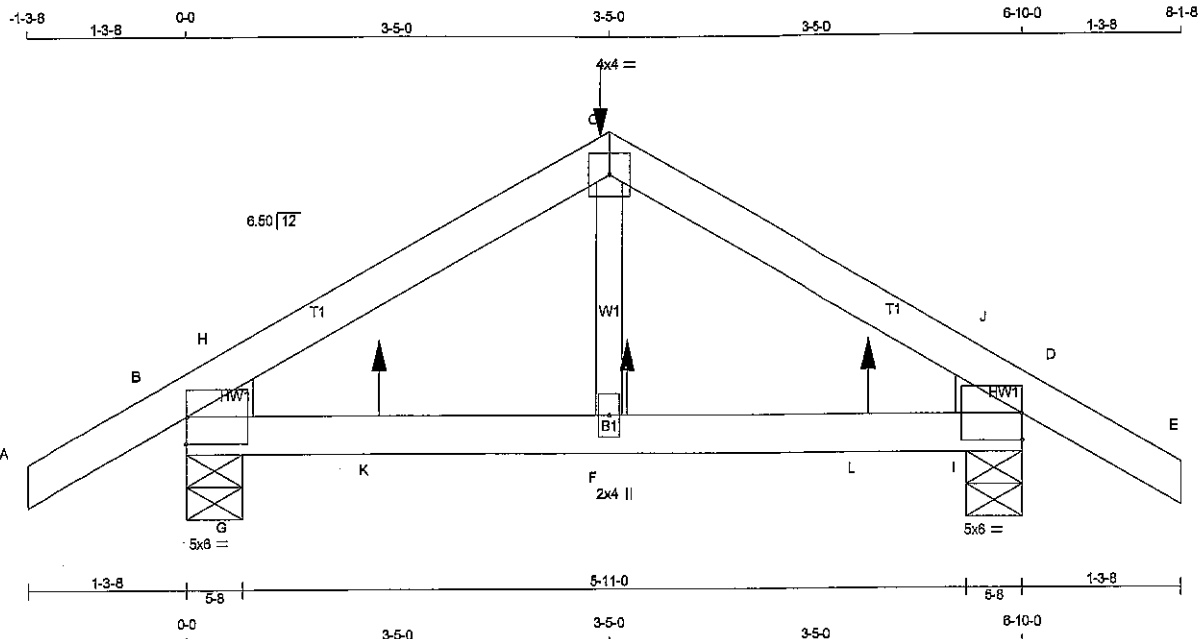


DWG NO. TAN 4607B17
STRUCTURAL
COMPONENT ONLY

JOB NAME 283746	TRUSS NAME T129Z	QUANTITY 1	PLY 1	JOB DESC. 43854	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID:Uier6HFwTUstfudnkBu50lkHylEuR-oyo9TIFeR2sqnMy7xWAusg_uKR4P0PjuDIPIAfyehYI



Scale = 1:17.8

TOTAL WEIGHT = 23 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-I	MT20	5.0	6.0		
C	TTW-p	MT20	4.0	4.0		
D	TMBH1-I	MT20	5.0	6.0		
F	BMW+w	MT20	2.0	4.0		

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 2.0 lbs FACTORED DOWN AND 103.3 lbs FACTORED UP AT 3-5-0 ON TOP CHORD, AND 15.9 lbs FACTORED UP AT 1-6-12, AND 15.9 lbs FACTORED UP AT 3-6-12, AND 15.9 lbs FACTORED UP AT 5-6-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG	HEEL WEDGE
	VERT	HORZ	DOWN	HORZ			
B	484	0	484	0	5-8	5-8	2x4 L
D	483	0	483	0	5-8	5-8	2x4 R

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	364	235 / 0	72 / 0	0 / 0	0 / 0	77 / 0	0 / 0
D	383	234 / 0	72 / 0	0 / 0	0 / 0	77 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 17	-84.3 -84.3	0.12 (1)	10.00	F-C	0 / 172	0.05 (7)
B-H	-405 / 0	-84.3 -84.3	0.08 (1)	6.25	G-H	-101 / 121	0.00 (1)
H-C	-380 / 0	-84.3 -84.3	0.11 (5)	6.25	I-J	-101 / 121	0.00 (1)
C-J	-360 / 0	-84.3 -84.3	0.11 (5)	6.25			
J-D	-405 / 0	-84.3 -84.3	0.08 (1)	6.25			
D-E	0 / 17	-84.3 -84.3	0.12 (1)	10.00			
B-G	0 / 309	-28.0 -28.0	0.11 (5)	10.00			
G-K	0 / 309	-28.0 -28.0	0.12 (1)	10.00			
K-F	0 / 309	-28.0 -28.0	0.12 (1)	10.00			
F-L	0 / 309	-28.0 -28.0	0.12 (1)	10.00			
L-I	0 / 309	-28.0 -28.0	0.12 (1)	10.00			
I-D	0 / 309	-28.0 -28.0	0.10 (5)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.
C	3-5-0	-1	-2	103	FRONT	VERT
F	3-6-12	10	---	16	FRONT	VERT
K	1-6-12	10	---	16	FRONT	VERT
L	5-6-12	10	---	16	FRONT	VERT

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 085-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.23")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CS1: TC=0.12 (D-E:1), BC=0.12 (F-G:1), WB=0.05 (C-F:7), SS1=0.11 (C-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

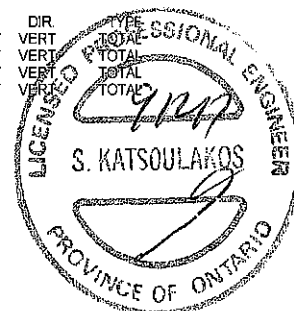
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	618	354	1667
	822	2284	1656

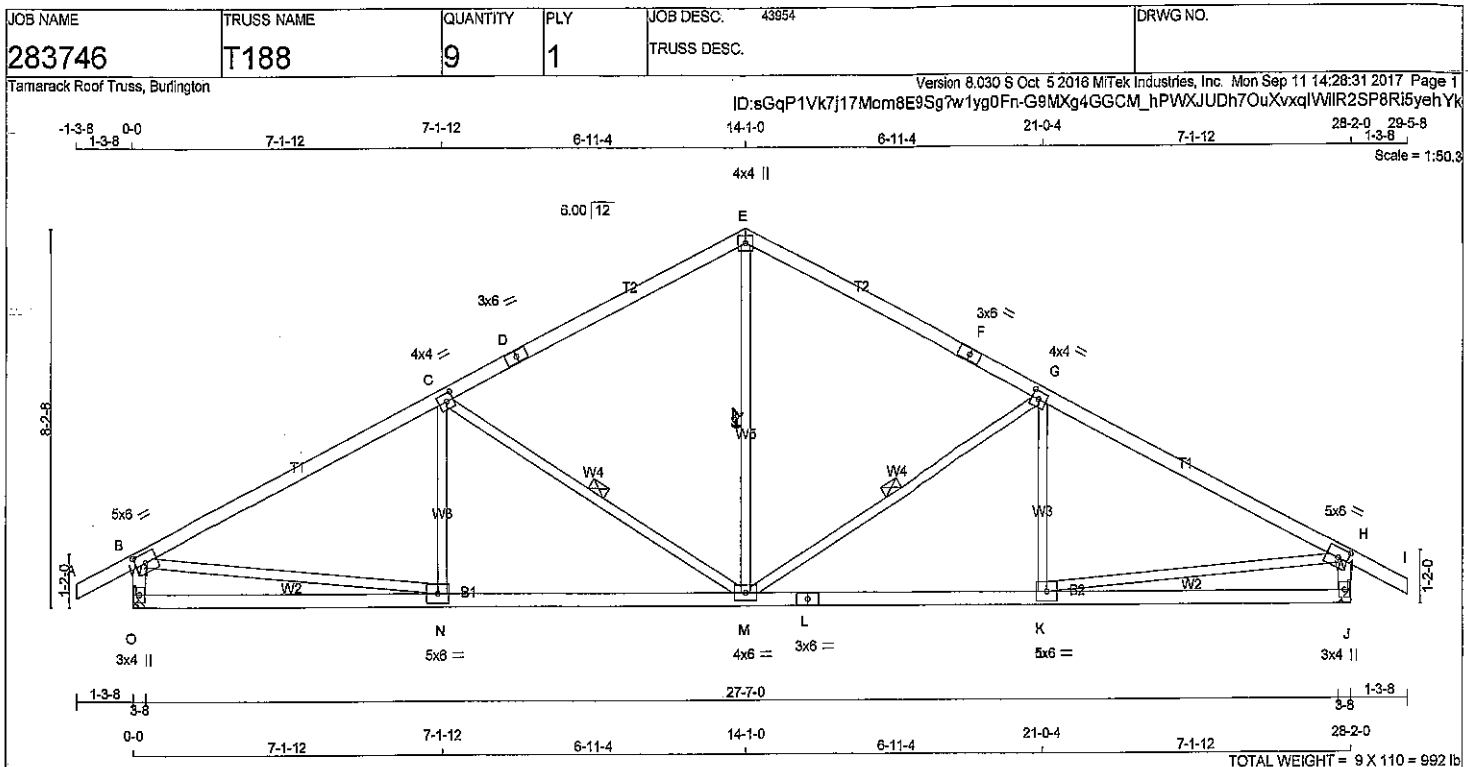
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (D) (INPUT = 0.90)
JSI METAL= 0.10 (D) (INPUT = 1.00)



DWG NO. TAM 46079-17
STRUCTURAL
COMPONENT ONLY



(M)

LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER			
N. L. G. A. RULES				BEARINGS			
CHORDS	SIZE	LUMBER	DESCR.	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
A - D	2x4	DRY	No.2	JT VERT	DOWN	IN-SX	
D - E	2x4	DRY	No.2	O 1895 0	1695 0	0	HANGER BY OTHERS
E - F	2x4	DRY	No.2				MIN. SEAT SIZE: 3-8
F - I	2x4	DRY	No.2				HANGER BY OTHERS
O - B	2x4	DRY	No.2	J 1895 0	1695 0	0	MIN. SEAT SIZE: 3-8
J - H	2x4	DRY	No.2				
O - L	2x4	DRY	No.2				
L - J	2x4	DRY	No.2				

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	2.75
C	TMVW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	6.0		
E	TTW+p	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMVW-t	MT20	4.0	4.0	2.00	1.75
H	TMVW-t	MT20	5.0	6.0	2.50	2.75
J	BMV1+p	MT20	3.0	4.0		
K	BMVW-t	MT20	5.0	6.0		
L	BS-t	MT20	3.0	6.0		
M	BMVW-t	MT20	4.0	6.0		
N	BMVW-t	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

UNFACTORED REACTIONS						
JT	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD
O	1376	790 / 0	295 / 0	0 / 0	0 / 0	290 / 0
J	1376	790 / 0	295 / 0	0 / 0	0 / 0	290 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.80 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-M, C-M. *EM*

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 26	-84.3 -84.3	0.11 (1)	10.00	M-E	0 / 982	0.22 (1)
B-C	-2254 / 0	-84.3 -84.3	0.70 (1)	3.80	M-G	-738 / 0	0.34 (1)
C-D	-1822 / 0	-84.3 -84.3	0.81 (1)	4.45	K-G	-27 / 269	0.08 (3)
D-E	-1822 / 0	-84.3 -84.3	0.81 (1)	4.45	C-M	-738 / 0	0.34 (1)
E-F	-1822 / 0	-84.3 -84.3	0.81 (1)	4.45	N-C	-27 / 269	0.08 (3)
F-G	-1822 / 0	-84.3 -84.3	0.81 (1)	4.45	B-N	0 / 2059	0.46 (1)
G-H	-2254 / 0	-84.3 -84.3	0.70 (1)	3.80	K-H	0 / 2059	0.46 (1)
H-I	0 / 26	-84.3 -84.3	0.11 (1)	10.00			
O-B	-1613 / 0	0.0	0.0 0.16 (1)	6.52			
J-H	-1613 / 0	0.0	0.0 0.16 (1)	6.52			
O-N	0 / 0	-28.0 -28.0	0.34 (3)	10.00			
N-M	0 / 2044	-28.0 -28.0	0.57 (2)	10.00			
M-L	0 / 2044	-28.0 -28.0	0.57 (2)	10.00			
L-K	0 / 2044	-28.0 -28.0	0.57 (2)	10.00			
K-J	0 / 0	-28.0 -28.0	0.34 (3)	10.00			

DESIGN CRITERIA			
SPECIFIED LOADS:			
TOP CH.	LL	=	25.6 PSF
	DL	=	3.0 PSF
BOT CH.	LL	=	10.5 PSF
	DL	=	7.0 PSF
TOTAL LOAD		=	46.1 PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.94")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.94")
CALCULATED VERT. DEFL.(TL) = L/ 998 (0.22")

CSI: TC=0.70 (B-C:1), BC=0.57 (M-N:2), WB=0.46 (B-N:1), SSI=0.26 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	618 354	1667 822	2284 1656

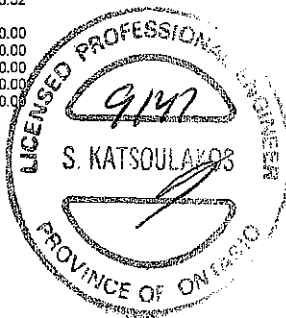
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (K) (INPUT = 0.90)
JSI METAL= 0.61 (H) (INPUT = 1.00)

DWG NO. TAM 46092-17

STRUCTURAL
COMPONENT ONLY



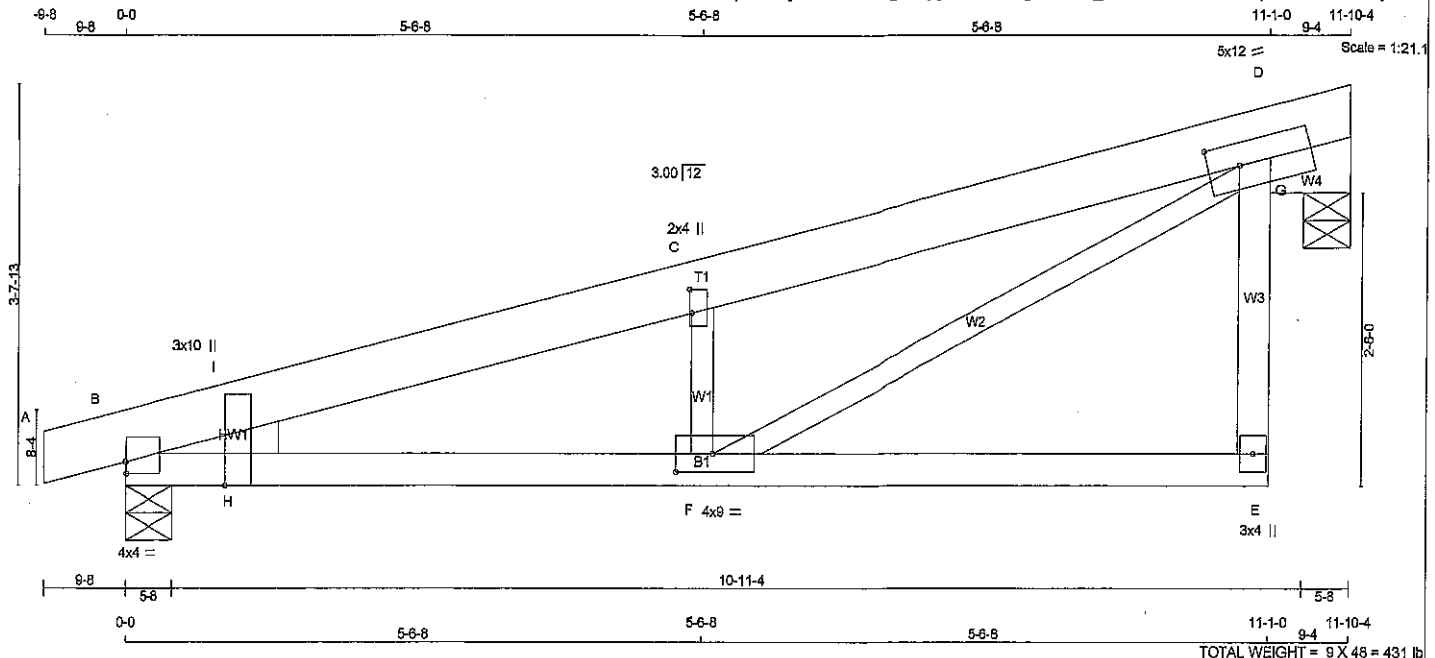
DWG NO. TAM 46092-17
STRUCTURAL
COMPONENT ONLY

JOB NAME 283746	TRUSS NAME T202TCX	QUANTITY 9	PLY 1	JOB DESC. 43854	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:sGqP1V7j17Mom8E9Sg?w1yg0Fn-G9MXg4GGCM_hPWxJUDh7OuX?AgMXloC2SP8Ri5yehYk



TOTAL WEIGHT = 9 X 48 = 431 lb

LUMBER			
N. E. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x6	DRY	No.2
E - D	2x4	DRY	No.2
B - E	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	4.0	4.0	1.25	
B	WP+I	MT20	3.0	10.0	Edge	11.50
C	TMW+u	MT20	2.0	4.0	2.50	0.25
D	TMVWW1-t	MT20	5.0	12.0	2.50	3.75
E	BMV+p	MT20	3.0	4.0		
F	BMWW-t	MT20	4.0	9.0	2.00	4.25

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG		HEEL WEDGE	
JT	VERT	JT	DOWN	JT	IN-SX	JT	IN-SX	JT	WEDGE
J(D)	593	593	0	5-8	5-8	5-8	5-8	2x4 L	
B	722	722	0	5-8	5-8	5-8	5-8		

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J(D)	487	270 / 0	111 / 0	0 / 0	0 / 0	108 / 0	0 / 0
B	583	340 / 0	122 / 0	0 / 0	0 / 0	121 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J(D), B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	-4 / 0	-84.3 -84.3	0.02 (1)	10.00	F-C	-460 / 0	0.07 (1)
B-I	-1371 / 0	-84.3 -84.3	0.04 (1)	6.25	F-D	0 / 1271	0.29 (1)
I-C	-1265 / 0	-84.3 -84.3	0.10 (1)	6.25	H-I	0 / 145	0.00 (1)
C-D	-1259 / 0	-84.3 -84.3	0.19 (1)	6.25	D-J	-828 / 0	0.03 (1)
E-G	0 / 113	0.0 0.0	0.37 (1)	10.00	G-J	0 / 578	0.00 (1)
G-D	0 / 113	0.0 0.0	0.37 (1)	10.00			
B-H	0 / 1231	-28.0 -28.0	0.22 (1)	10.00			
H-F	0 / 1231	-28.0 -28.0	0.32 (2)	10.00			
F-E	0 / 104	-28.0 -28.0	0.19 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. G/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.37")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.06")
ALLOWABLE DEFL.(TL) = $L/360$ (0.37")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.11")

CSI: TC=0.37 (E-G:1), BC=0.32 (F-H:2), WB=0.29 (D-F:1), SS=0.16 (D-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	618 354	1667 822	2264 1656

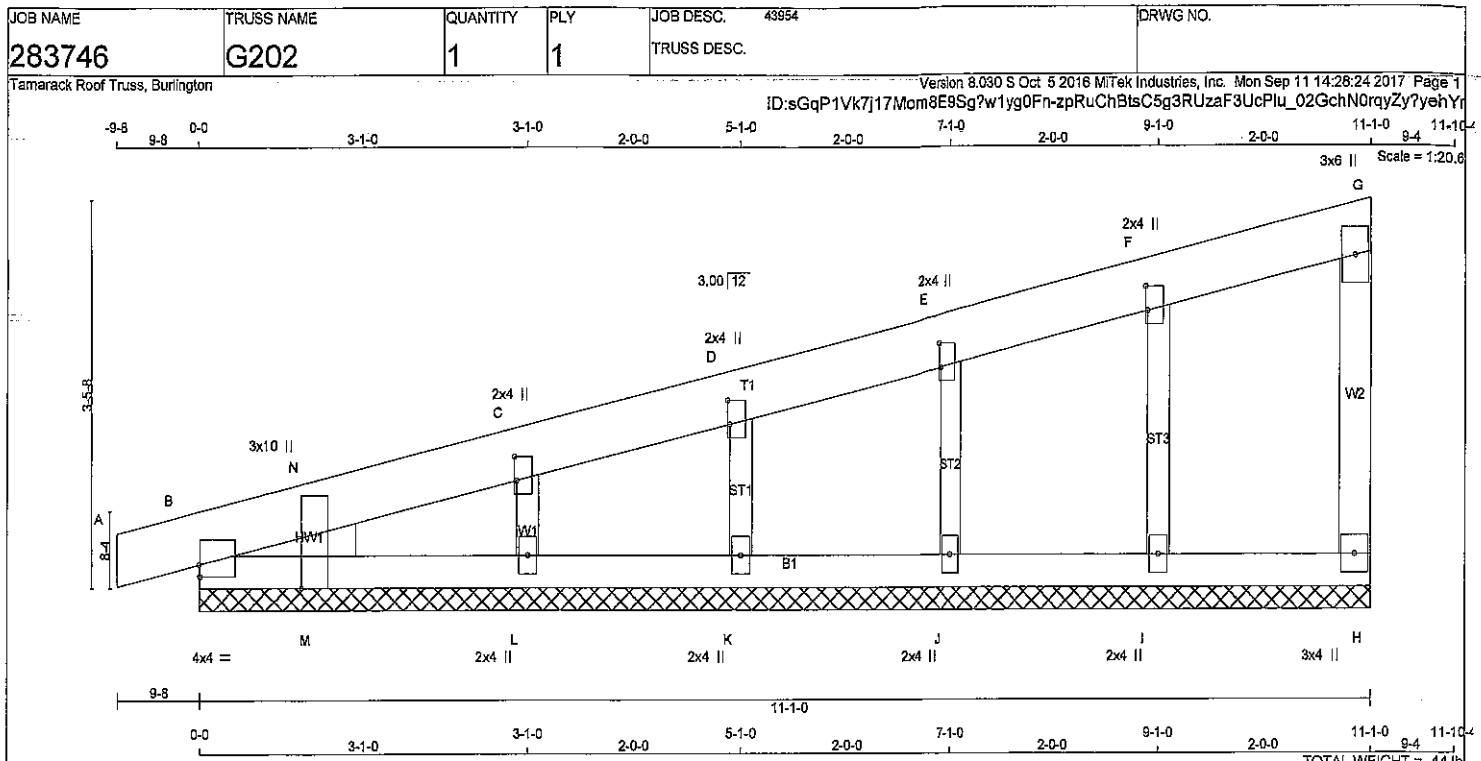
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (F) (INPUT = 0.90)
JSI METAL= 0.44 (B) (INPUT = 1.00)



DWG NO. TAM 46081-17
STRUCTURAL
COMPONENT ONLY



LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - G	2x6	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
B - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

GABLE STUDS SPACED AT 2'-0" OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-L	MT20	4.0	4.0	1.25	
C	WP-H	MT20	3.0	10.0	Edge	11.50
C, D, E, F						
C	TMW+H	MT20	2.0	4.0	2.50	0.25
G	TMV+P	MT20	3.0	6.0		
H	BMV1+P	MT20	3.0	4.0		
I, J, K, L						
I	BMW1+H	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

HEEL
WEDGE
2x4 L
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX LC1 (LC)	
FR-TO		FROM TO		FR-TO			
A-B	-41.0	-84.3	-84.3 0.02 (1)	I-F	-185.0	0.03 (1)	
B-N	-57.0	-84.3	-84.3 0.04 (1)	J-E	-171.0	0.03 (1)	
N-C	-11.4	-84.3	-84.3 0.05 (1)	K-D	-139.0	0.02 (1)	
C-D	-20.0	-84.3	-84.3 0.05 (1)	L-C	-256.0	0.04 (1)	
D-E	-8.0	-84.3	-84.3 0.02 (1)	M-N	0.0	0.00 (1)	
E-F	-4.0	-84.3	-84.3 0.02 (1)				
F-G	-4.0	-84.3	-84.3 0.02 (1)				
H-G	-72.0	0.0	0.0 0.01 (1)				
B-M	0.0	-28.0	-28.0 0.03 (1)				
M-L	0.0	-28.0	-28.0 0.04 (2)				
L-K	0.0	-28.0	-28.0 0.04 (2)				
K-J	0.0	-28.0	-28.0 0.02 (2)				
J-I	0.0	-28.0	-28.0 0.03 (2)				
I-H	0.0	-28.0	-28.0 0.03 (3)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. GIC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.05 (C-D-1), BC=0.04 (L-M-2), WB=0.04 (C-L-1), SSI=0.09 (C-N-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	618	354	1887 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (C) (INPUT = 0.90)
JSI METAL= 0.05 (L) (INPUT = 1.00)

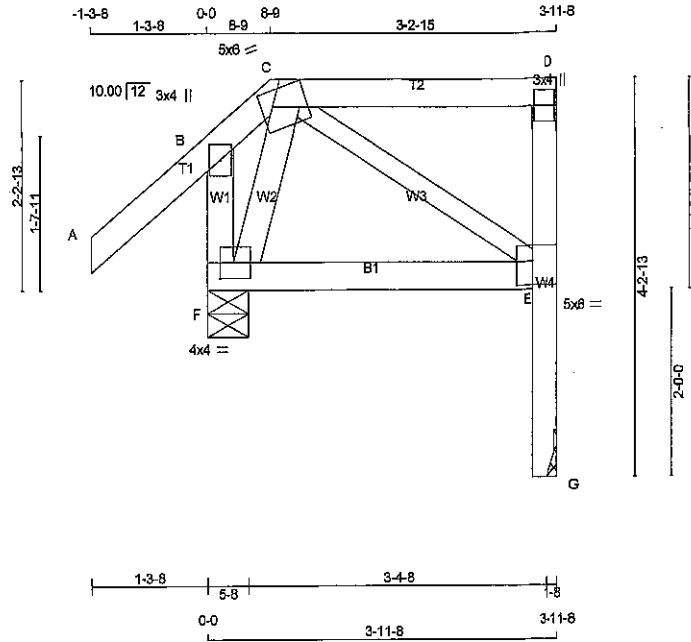


DWG NO. TAM 46145-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43854	DRWG NO.
283746	T210	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1:24.6

TOTAL WEIGHT = 2 X 22 = 43 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
G - D	2x4	DRY	No.2	SPF	
F - B	2x4	DRY	No.2	SPF	
F - E	2x4	DRY	No.2	SPF	
ALL WEBS	2x4	DRY	No.2	SPF	
EXCEPT					
C - E	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TTWW-m	MT20	5.0	6.0	2.00	1.50
D	TMV+p	MT20	3.0	4.0		
E	BVMW-l	MT20	5.0	6.0	3.00	2.50
F	BMVW-l	MT20	4.0	4.0	2.00	1.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG
	VERT	HORZ	DOWN	HORZ
G	202	0	202	0
F	359	0	359	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN	COMPONENT REACTIONS	WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE		
G	169	89 / -12	42 / 0	0 / 0	39 / 0	0 / 0
F	275	185 / 0	42 / 0	0 / 0	49 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1 (PLF)	MEMB.	FORCE (LBS)
FR-TO		FROM TO	FR-TO	
A-B	0 / 37	-84.3 -84.3 0.12 (1)	10.00	F-C -54 / 81 0.01 (5)
B-C	-86 / 0	-84.3 -84.3 0.11 (1)	6.25	C-E -24 / 35 0.01 (5)
C-D	0 / 0	-84.3 -84.3 0.15 (1)	10.00	
G-E	-202 / 0	0.0 0.0 0.03 (1)	7.81	
E-D	-134 / 0	0.0 0.0 0.02 (1)	7.81	
F-B	-253 / 0	0.0 0.0 0.03 (1)	7.81	
F-E	-30 / 20	-28.0 -28.0 0.13 (3)	6.25	

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 089.09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.15 (C-D:1), BC=0.13 (E-F:3), WB=0.01 (C-F:5), SSI=0.10 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (F) (INPUT = 0.90)
JSI METAL= 0.07 (C) (INPUT = 1.00)



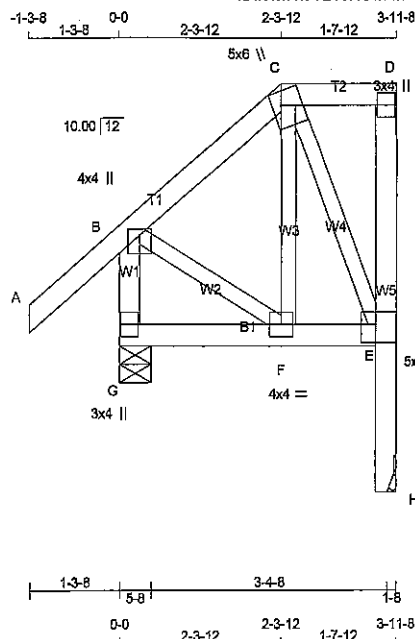
DWG NO. TAM 46002-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
283746	T211	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1:31.5

TOTAL WEIGHT = 2 X 28 = 52 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	DRY
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
H - D	2x4	DRY	No.2
G - B	2x4	DRY	No.2
G - E	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMV+p	MT20	3.0	4.0		
E	BVMW-l	MT20	5.0	6.0	3.00	2.50
F	BMWW-t	MT20	4.0	4.0		
G	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REORD BRG
	VERT	HORZ	DOWN	HORZ		
H	222	0	222	0	IN-SX	IN-SX
G	339	0	339	0	HANGER BY OTHERS	MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX/MIN. LIVE		WIND	DEAD	SOIL
	SNOW	LIVE	PERM.LIVE	WIND			
H	182	101/0	42/0	0/0	0/0	40/0	0/0
G	262	172/0	42/0	0/0	0/0	48/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS		FACTORED		WEBS		MAX. FACTORED	
	FORCE (LBS)	VERT. LOAD LC1	MAX (PLF)	MAX (LC)	MEMB.	FORCE (LBS)	MAX (LBS)	MAX (LC)
FR-TO					FR-TO			
A-B	0/37	-84.3	-84.3	0.12 (1)	10.00	F-C	0/100	0.02 (3)
B-C	-90/0	-84.3	-84.3	0.06 (1)	6.25	C-E	-154/0	0.03 (1)
C-D	0/0	-84.3	-84.3	0.04 (1)	10.00	B-F	0/78	0.02 (1)
H-E	-222/0	0.0	0.0	0.03 (1)	7.81			
E-D	-69/0	0.0	0.0	0.02 (1)	7.81			
G-B	-309/0	0.0	0.0	0.03 (1)	7.81			
G-F	0/0	-28.0	-28.0	0.04 (2)	10.00			
F-E	0/70	-28.0	-28.0	0.04 (2)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.12 (A-B:1), BC=0.04 (E-F:2), WB=0.03 (C-E:1), SS=0.07 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 615 354 1687 822 2284 1556

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)



DWG NO. TAN 4608317
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
283746	T240	2	1	43954		

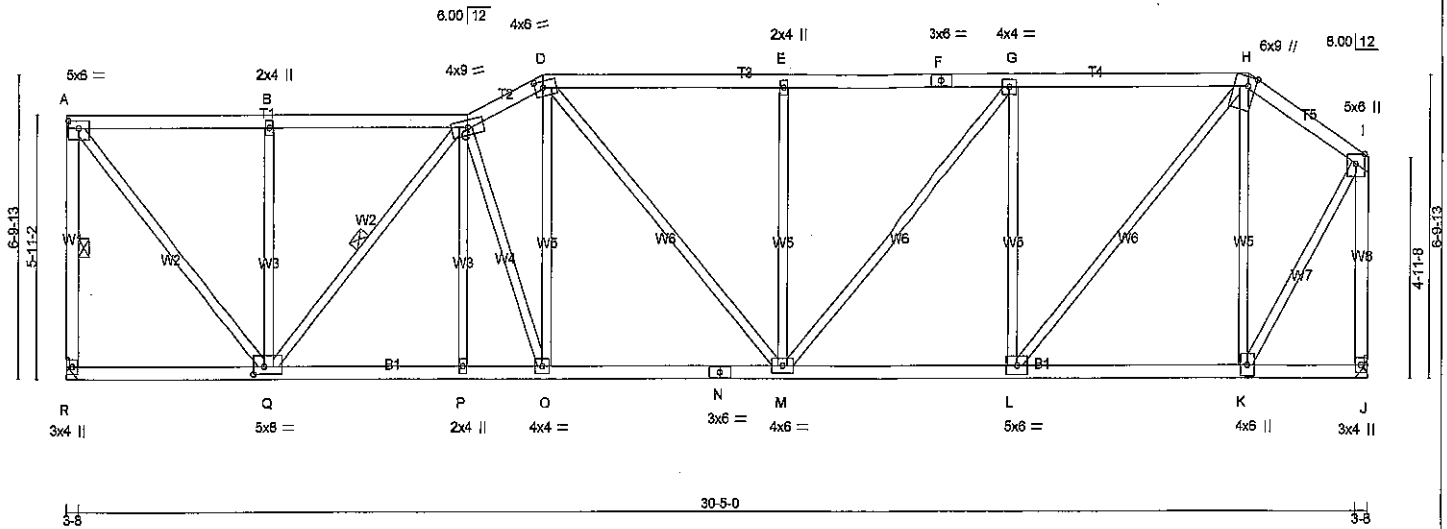
Tamarack Roof Truss, Burlington

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0-0 4-9-2 4-9-2 4-9-2 9-6-5 11-3-10 5-8-1 16-11-11 5-6-13 22-6-7 5-8-1 28-2-8 2-9-8 31-0-0

Scale = 1:51.8



TOTAL WEIGHT = 2 X 152 = 305 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
R - A	2x4	DRY	No.2
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - I	2x4	DRY	No.2
J - I	2x4	DRY	No.2
R - N	2x4	DRY	No.2
N - J	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
SPF			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW-t	MT20	5.0	6.0	2.00 3.00
B	TMVW-w	MT20	2.0	4.0	
C	TTWWVW-m	MT20	4.0	9.0	
D	TTWWVW-m	MT20	4.0	6.0	1.75 2.25
E	TMVW-w	MT20	2.0	4.0	
F	TS-t	MT20	3.0	6.0	
G	TMVW-t	MT20	4.0	4.0	
H	TTWWVW-m	MT20	6.0	9.0	Edge 2.00
I	TMVW-p	MT20	5.0	6.0	Edge
J	BMV1+p	MT20	3.0	4.0	
K	BMVW-t	MT20	4.0	6.0	
L	BMVW-t	MT20	5.0	6.0	
M	BMVWVW-t	MT20	4.0	6.0	
N	BS-t	MT20	3.0	6.0	
O	BMVW-t	MT20	4.0	4.0	
P	BMVW-w	MT20	2.0	4.0	
Q	BMVWVW-t	MT20	5.0	8.0	2.00 3.00
R	BMV1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT REGRD	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
R	1740	0	1740	0	0	HANGER BY OTHERS	MIN. SEAT SIZE: 3-8
J	1740	0	1740	0	0	HANGER BY OTHERS	MIN. SEAT SIZE: 3-8

UNFACTORED REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
R	1429	793 / 0	326 / 0	0 / 0	0 / 0	310 / 0
J	1429	793 / 0	326 / 0	0 / 0	0 / 0	310 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF A-R, C-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

CHORDS		FACTORED		WEBS	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)
FR-TO		FROM	TO	FR-TO	
R-A	-1688 / 0	0.0	0.0 0.29 (1)	A-Q	0 / 2010
A-B	-1304 / 0	-84.3	-84.3 0.35 (1)	Q-B	-493 / 0
B-C	-1304 / 0	-84.3	-84.3 0.35 (1)	C-Q	-1147 / 0
C-D	-2146 / 0	-84.3	-84.3 0.09 (1)	D-C	0 / 109
D-E	-2088 / 0	-84.3	-84.3 0.47 (1)	E-D	-380 / 0
E-F	-2088 / 0	-84.3	-84.3 0.47 (1)	F-E	0 / 493
F-G	-2088 / 0	-84.3	-84.3 0.47 (1)	G-F	-991 / 0
G-H	-1890 / 0	-84.3	-84.3 0.44 (1)	H-G	0 / 1331
H-I	-849 / 0	-84.3	-84.3 0.12 (1)	I-H	0 / 1524
I-J	-1720 / 0	0.0	0.0 0.75 (1)	J-I	0 / 258
R-Q	0 / 0	-28.0	-28.0 0.17 (3)	Q-R	-992 / 0
Q-P	0 / 2041	-28.0	-28.0 0.41 (1)	P-Q	-514 / 0
P-O	0 / 2040	-28.0	-28.0 0.40 (1)	O-P	0 / 626
O-N	0 / 1927	-28.0	-28.0 0.40 (2)	N-O	
N-M	0 / 1927	-28.0	-28.0 0.40 (2)	M-N	
M-L	0 / 1690	-28.0	-28.0 0.37 (2)	L-M	
L-K	0 / 689	-28.0	-28.0 0.25 (2)	K-L	
K-J	0 / 0	-28.0	-28.0 0.14 (2)	J-K	

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/999 (0.13")
ALLOWABLE DEFL.(TL) = L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/999 (0.21")

CSI: TC=0.75 (I-J), BC=0.41 (P-Q), WB=0.77 (G-L), SS=0.22 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

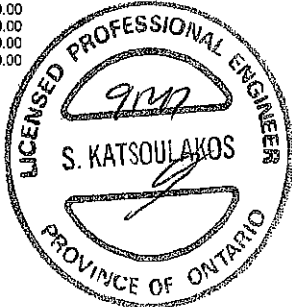
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (O) (INPUT = 0.90)
JSI METAL= 0.67 (C) (INPUT = 1.00)



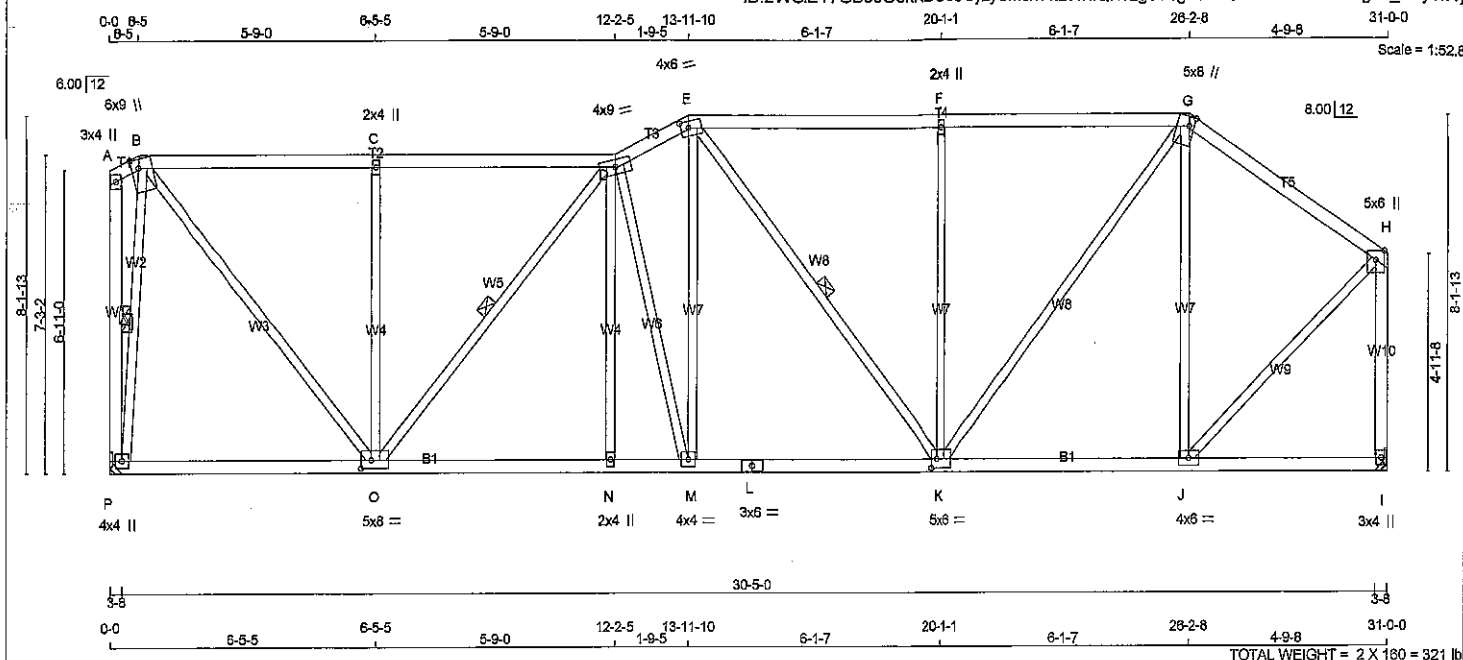
DRWG NO. TAM-4608417
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43854	DRWG NO.
283746	T241	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:2WGIEY7GB80O3KKB0Cc5zyemsm-kLvwuQHvzg6Y1g5V2wCMx5447ErfU5sBg3u_ExYehYj



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - B	2x4	DRY	No.2
B - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - H	2x4	DRY	No.2
P - A	2x4	DRY	No.2
I - H	2x4	DRY	No.2
P - L	2x4	DRY	No.2
L - I	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
A	TMV+p	MT20	3.0	4.0
B	TTWW+m	MT20	6.0	9.0 2.50 2.00
C	TMW+w	MT20	2.0	4.0
D	TTWWW-m	MT20	4.0	9.0
E	TTWW-m	MT20	4.0	5.0 1.75 2.25
F	TMW+w	MT20	2.0	4.0
G	TTWW+m	MT20	5.0	6.0 2.50 1.50
H	TMW+w	MT20	5.0	6.0 Edge
I	BMV1+p	MT20	3.0	4.0
J	BMW-t	MT20	4.0	6.0
K	BMWW-t	MT20	5.0	6.0 2.50 1.50
L	BS-t	MT20	3.0	6.0
M	BMW-t	MT20	4.0	4.0
N	BMW+w	MT20	2.0	4.0
O	BMWW-t	MT20	5.0	8.0 2.25 3.00
P	BMW1+p	MT20	4.0	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT		REQD	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	HANGER BY OTHERS	MIN. SEAT SIZE: 3-8
P	1740	0	1740	0	0			HANGER BY OTHERS	MIN. SEAT SIZE: 3-8
I	1740	0	1740	0	0			HANGER BY OTHERS	MIN. SEAT SIZE: 3-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1429	793 / 0	326 / 0	0 / 0	0 / 0	310 / 0	0 / 0
I	1429	793 / 0	326 / 0	0 / 0	0 / 0	310 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.37 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-O, E-K, A-P, B-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 0	-84.3 -84.3	0.01 (1)	10.00	O-C	-597 / 0	0.55 (1)
B-C	-1338 / 0	-84.3 -84.3	0.52 (1)	4.89	O-D	-806 / 0	0.45 (1)
C-D	-1338 / 0	-84.3 -84.3	0.52 (1)	4.89	N-D	0 / 128	0.03 (3)
D-E	-1902 / 0	-84.3 -84.3	0.08 (1)	4.82	D-M	-573 / 0	0.58 (1)
E-F	-1624 / 0	-84.3 -84.3	0.63 (1)	4.37	M-E	0 / 686	0.15 (1)
F-G	-1624 / 0	-84.3 -84.3	0.63 (1)	4.37	E-K	-140 / 0	0.10 (1)
G-H	-1107 / 0	-84.3 -84.3	0.37 (1)	5.53	K-F	-634 / 0	0.82 (1)
P-A	-29 / 0	0.0 0.0	0.01 (1)	6.25	K-G	0 / 1160	0.26 (1)
I-H	-1689 / 0	0.0 0.0	0.74 (1)	6.39	J-G	-700 / 0	0.90 (1)
					J-H	0 / 1267	0.29 (1)
P-O	0 / 164	-28.0 -28.0	0.30 (2)	10.00	P-B	-1648 / 0	0.55 (1)
O-N	0 / 1852	-28.0 -28.0	0.50 (2)	10.00	B-O	0 / 1845	0.42 (1)
N-M	0 / 1851	-28.0 -28.0	0.38 (1)	10.00			
M-L	0 / 1710	-28.0 -28.0	0.41 (2)	10.00			
L-K	0 / 1710	-28.0 -28.0	0.41 (2)	10.00			
K-J	0 / 911	-28.0 -28.0	0.32 (2)	10.00			
J-I	0 / 0	-28.0 -28.0	0.20 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN./C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/999 (0.18")

CSI: TC=0.74 (H-I:1), BC=0.50 (N-O:2), WB=0.90 (G-J:1), SI=0.25 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

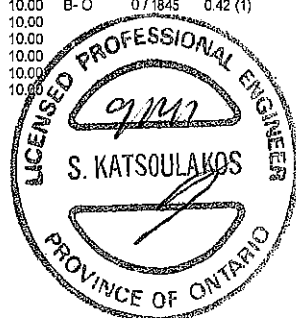
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL= 0.54 (D) (INPUT = 1.00)



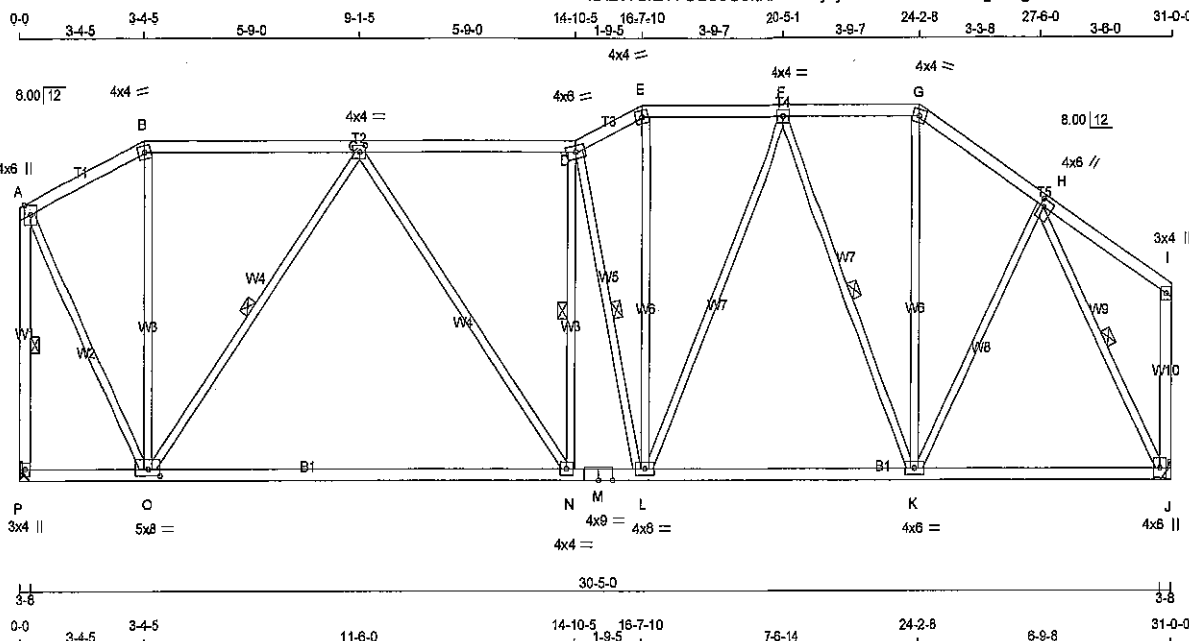
DWB NB, FAM 46085-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
283746	T242	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1:58.6

TOTAL WEIGHT = 2 X 170 = 339 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2
B - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - J	2x4	DRY	No.2
J - L	2x4	DRY	No.2
L - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
SPF			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW+p	MT20	4.0	6.0	Edge
B, E, G					
B	TTW-m	MT20	4.0	4.0	
C	TMWW-t	MT20	4.0	4.0	2.00 1.75
D	TTWW-m	MT20	4.0	6.0	
F	TMWW-t	MT20	4.0	4.0	
H	TMWW-t	MT20	4.0	6.0	2.25 1.25
I	TMV+p	MT20	3.0	4.0	
J	BMVW1+p	MT20	4.0	6.0	
K	BMWW-t	MT20	4.0	6.0	
L	BMWW-t	MT20	4.0	6.0	
M	BS-t	MT20	4.0	9.0	
N	BMWW-t	MT20	4.0	4.0	
O	BMWW-t	MT20	5.0	8.0	2.00 4.00
P	BMV1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
P	1740	0	1740	0	0	
J	1740	0	1740	0	0	

UNFACTORED REACTIONS

JT	1ST CASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE				
P	1429	793 / 0	326 / 0	0 / 0	0 / 0	310 / 0	0 / 0
J	1429	793 / 0	326 / 0	0 / 0	0 / 0	310 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.46 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-O, D-N, D-L, F-K, A-P, H-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	MAX. LC1 (LC)
FR-TO				FR-TO			
A-B	-752 / 0	-84.3	-84.3 0.17 (1)	Q-B	0 / 158	0.04 (3)	
B-C	-672 / 0	-84.3	-84.3 0.49 (1)	Q-C	-1233 / 0	0.91 (1)	
C-D	-1851 / 0	-84.3	-84.3 0.56 (1)	C-N	0 / 508	0.11 (1)	
D-E	-1630 / 0	-84.3	-84.3 0.07 (1)	N-D	-134 / 167	0.06 (1)	
E-F	-1464 / 0	-84.3	-84.3 0.21 (1)	D-L	-877 / 0	0.44 (1)	
F-G	-1032 / 0	-84.3	-84.3 0.20 (1)	L-E	0 / 527	0.12 (1)	
G-H	-1254 / 0	-84.3	-84.3 0.16 (1)	L-F	0 / 375	0.08 (1)	
H-I	0 / 17	-84.3	-84.3 0.14 (1)	F-K	-787 / 0	0.58 (1)	
P-A	-1762 / 0	0.0	0.0 0.39 (1)	K-G	0 / 447	0.10 (1)	
J-I	-111 / 0	0.0	0.0 0.05 (1)	K-H	0 / 594	0.13 (1)	
				A-O	0 / 1448	0.33 (1)	
P-O	0 / 0	-28.0	-28.0 0.59 (3)	H-J	-1731 / 0	0.68 (1)	
O-N	0 / 1365	-28.0	-28.0 0.78 (2)				
N-M	0 / 1652	-28.0	-28.0 0.83 (2)				
M-L	0 / 1652	-28.0	-28.0 0.83 (2)				
L-K	0 / 1325	-28.0	-28.0 0.53 (2)				
K-J	0 / 781	-28.0	-28.0 0.47 (2)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL)= L/981 (0.38")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL)= L/592 (0.63")

CSI: TC=0.56 (C-D:1), BC=0.83 (L-N:2), WB=0.91 (C-O:1), SS=0.24 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 622 2294 1656

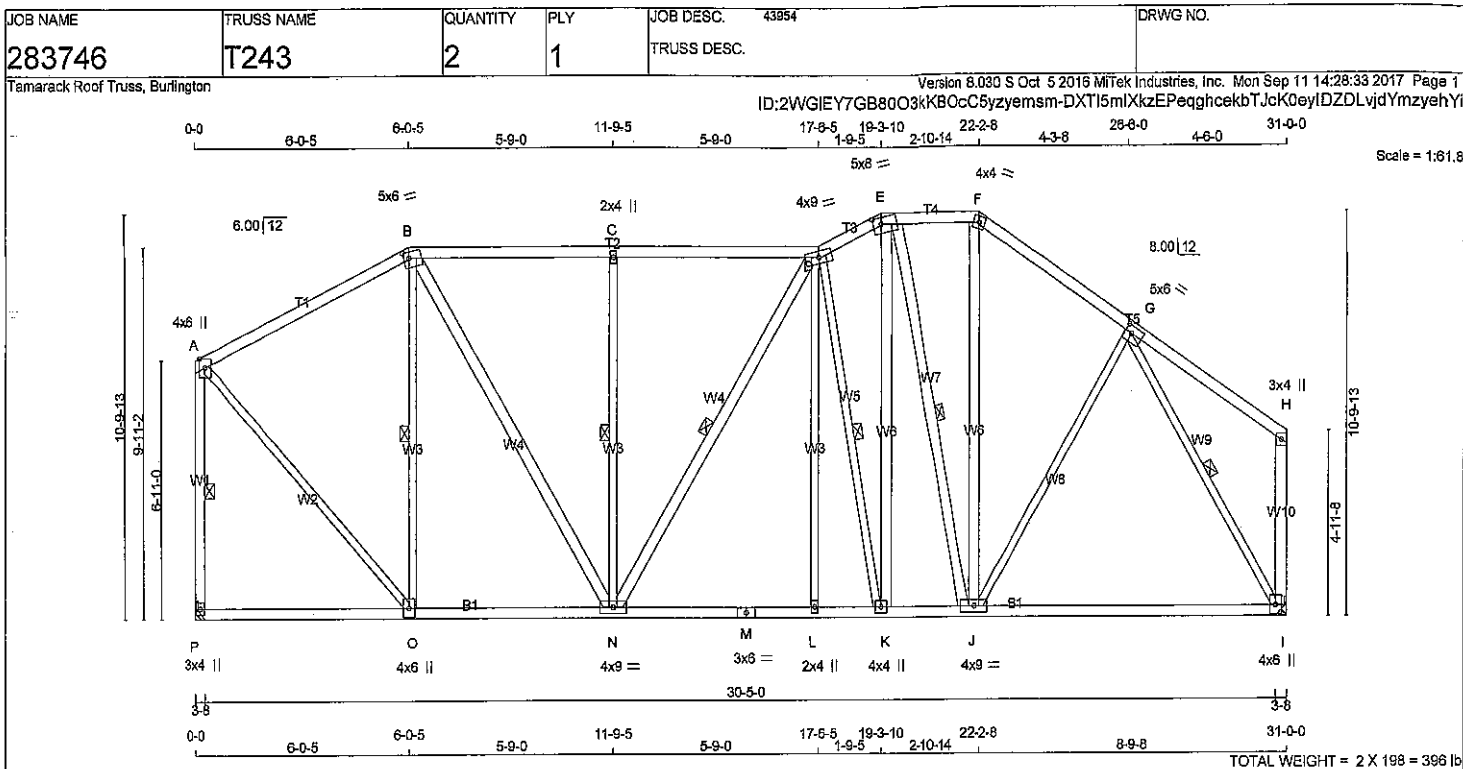
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (H) (INPUT = 0.90)
JSI METAL= 0.88 (M) (INPUT = 1.00)



DRWG NO. TAM 4609617
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2
B - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
P - A	2x4	DRY	No.2
I - H	2x4	DRY	No.2
P - M	2x4	DRY	No.2
M - I	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
B - N	2x4	DRY	No.2
N - D	2x4	DRY	No.2
K - E	2x4	DRY	No.2
E - J	2x4	DRY	No.2
J - F	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
B	TTWW-m	MT20	5.0	6.0	2.00	1.75
C	TMW+w	MT20	2.0	4.0		
D	TTWWW-m	MT20	4.0	9.0		
E	TTWW-m	MT20	5.0	8.0	2.00	2.75
F	TTW-m	MT20	4.0	4.0		
G	TMWWW-t	MT20	5.0	6.0	2.25	2.25
H	TMV+p	MT20	3.0	4.0		
I	BMVW1+p	MT20	4.0	6.0		
J	BMWWW-t	MT20	4.0	9.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMW+w	MT20	2.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	9.0		
O	BMWW-t	MT20	4.0	6.0		
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQD BRG IN-SX
P	1740	0	1740	0	0	HANGER BY OTHERS	MIN. SEAT SIZE: 3-8
I	1740	0	1740	0	0	HANGER BY OTHERS	MIN. SEAT SIZE: 3-8

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	MAX / MIN. SNOW	COMPONENT REACTIONS LIVE	PERM. LIVE	WIND	DEAD	SOIL
P	1429	793 / 0	326 / 0	0 / 0	0 / 0	310 / 0	0 / 0
I	1429	793 / 0	326 / 0	0 / 0	0 / 0	310 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.11 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF B-O, C-N, D-N, D-K, E-J, A-P, G-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 MAX (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	MAX. FACTORED FORCE (LBS)
FR-TO				FR-TO			
A-B	-1004 / 0	-84.3	-84.3 0.41 (1)	O-B	-779 / 0	5.69	0.52 (1)
B-C	-1360 / 0	-84.3	-84.3 0.38 (1)	B-N	0 / 919	5.11	0.15 (1)
C-D	-1390 / 0	-84.3	-84.3 0.38 (1)	N-C	-597 / 0	5.11	0.40 (1)
D-E	-1327 / 0	-84.3	-84.3 0.05 (1)	N-D	-54 / 0	5.58	0.04 (2)
E-F	-1073 / 0	-84.3	-84.3 0.10 (1)	L-D	0 / 273	5.98	0.05 (3)
F-G	-1306 / 0	-84.3	-84.3 0.22 (1)	D-K	-987 / 0	5.42	0.69 (1)
G-H	0 / 24	-84.3	-84.3 0.28 (1)	K-E	0 / 879	10.00	0.14 (1)
P-A	-1671 / 0	0.0	0.0 0.37 (1)	E-J	-498 / 0	5.14	0.31 (1)
I-H	-141 / 0	0.0	0.0 0.05 (1)	J-F	0 / 447	7.81	0.07 (1)
				J-G	0 / 384		0.09 (1)
				A-O	0 / 1319		0.30 (1)
P-O	0 / 0	-28.0	-28.0 0.25 (2)	G-I	-1733 / 0		0.89 (1)
O-N	0 / 889	-28.0	-28.0 0.35 (2)				
N-M	0 / 1382	-28.0	-28.0 0.33 (2)				
M-L	0 / 1382	-28.0	-28.0 0.33 (2)				
L-K	0 / 1380	-28.0	-28.0 0.29 (1)				
K-J	0 / 1199	-28.0	-28.0 0.58 (2)				
J-I	0 / 868	-28.0	-28.0 0.84 (2)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA C86-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/999 (0.23")
ALLOWABLE DEFL.(TL) = L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/969 (0.38")

CSI: TC=0.41 (A-B:1), BC=0.68 (J-K:2), WB=0.89 (G-I:1), SSI=0.23 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1887 822 2284 1656

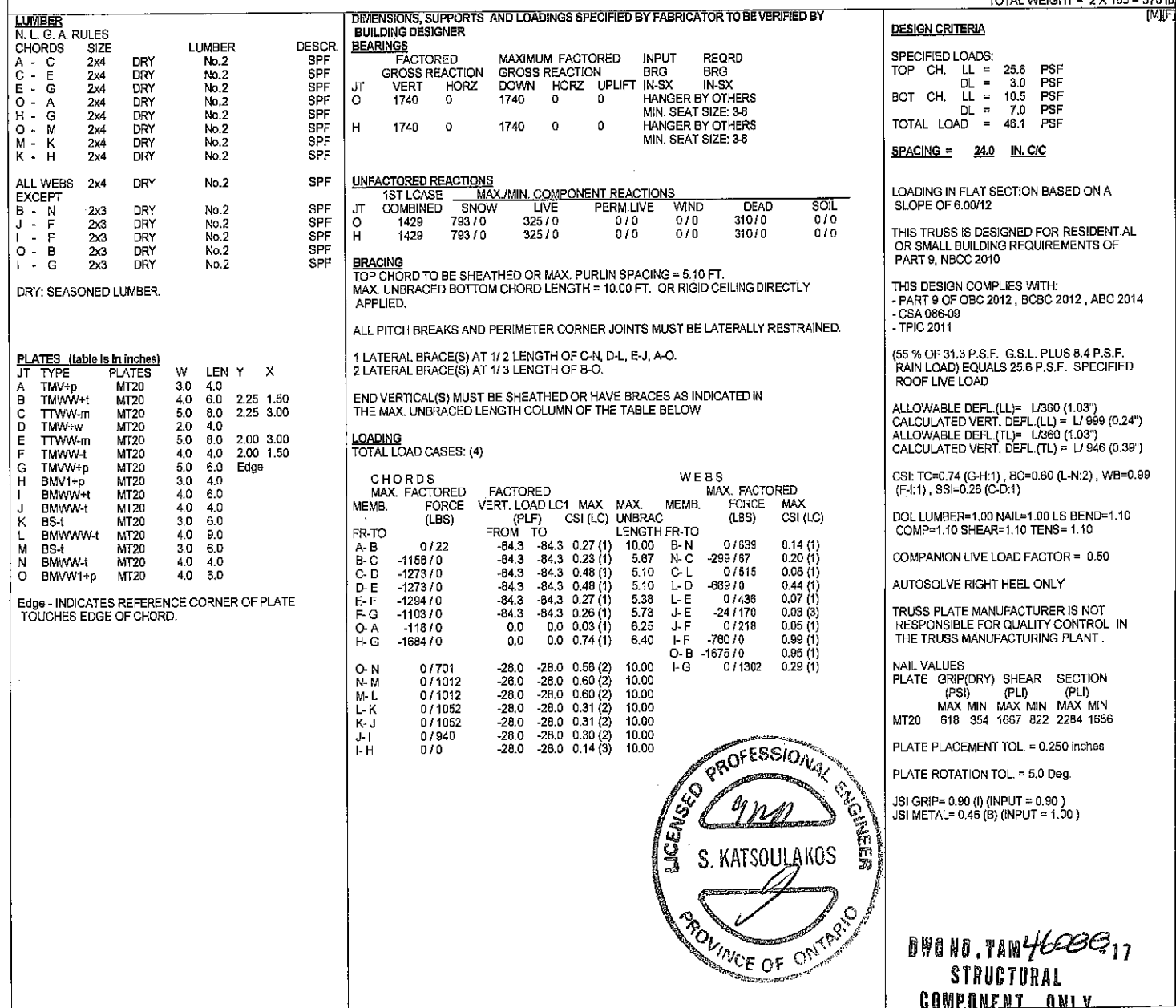
PLATE PLACEMENT TOL. = 0.250 inches

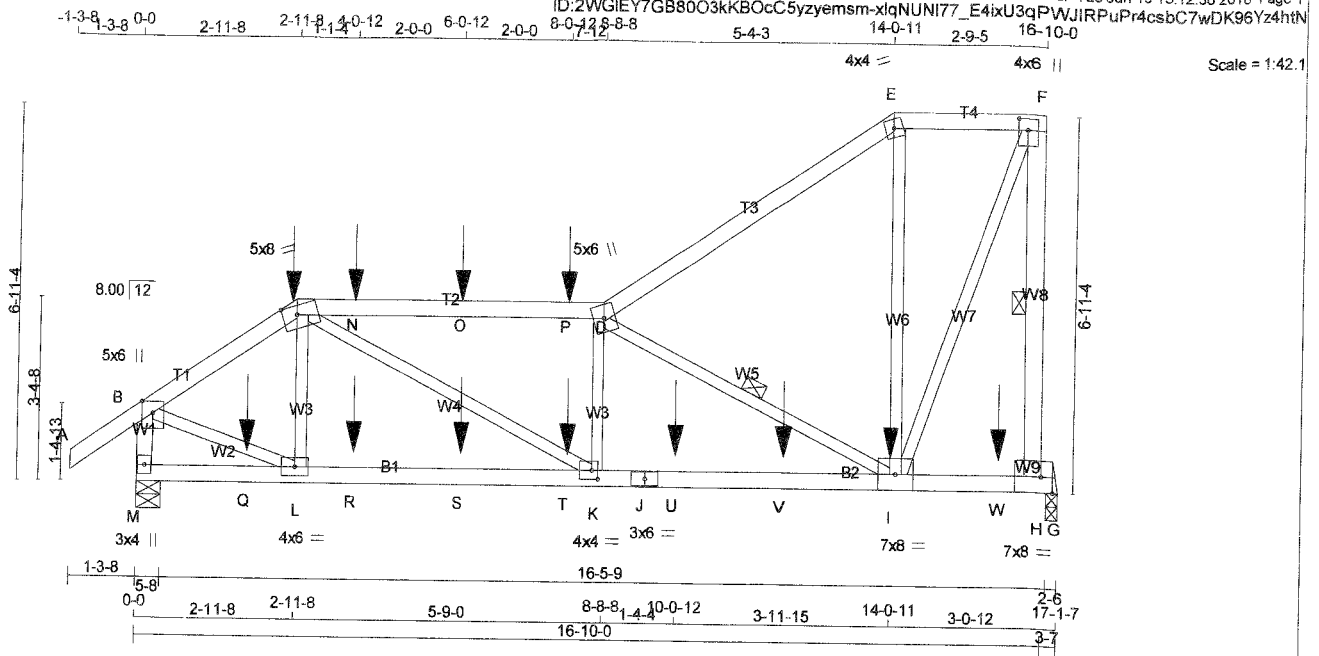
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (G) (INPUT = 0.90)
JSI METAL= 0.46 (M) (INPUT = 1.00)



DRW NO. TAM 46087-1
STRUCTURAL COMPONENT ONLY





LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x4	DRY	2100F 1.8E	SPF
J - G	2x4	DRY	2100F 1.8E	SPF
H - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW-m	MT20	5.0	8.0	2.00	3.25
D	TTWW-m	MT20	5.0	6.0		
E	TTW-m	MT20	4.0	4.0		
F	TMVW+p	MT20	4.0	6.0	2.50	2.00
H	BMVW-t	MT20	7.0	8.0	Edge	2.50
I	BMVW-t	MT20	7.0	8.0		
J	BS-t	MT20	3.0	6.0		
K	BMVW-t	MT20	4.0	4.0	2.00	1.50
L	BMVW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 75.0 lbs FACTORED DOWN AT 2-11-8, 9.4 lbs FACTORED DOWN AT 4-0-12, AND 9.4 lbs FACTORED DOWN AT 6-0-12, AND 24.4 lbs FACTORED DOWN AT 8-0-12 ON TOP CHORD, AND 14.8 lbs FACTORED DOWN AT 2-0-12, 14.8 lbs FACTORED DOWN AT 4-0-12, 14.8 lbs FACTORED DOWN AT 6-0-12, 14.8 lbs FACTORED DOWN AT 8-0-12, 173.8 lbs FACTORED DOWN AT 10-0-12, 194.2 lbs FACTORED DOWN AT 12-0-12, AND 194.2 lbs FACTORED DOWN AT 14-0-12, AND 194.2 lbs FACTORED DOWN AT 16-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
M	1363	0	0/0	5-8
G	1559	0	0/0	2-6

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
M	1090	652 / 0	217 / 0	0 / 0	0 / 0	222 / 0	0 / 0	0 / 0
G	1248	744 / 0	250 / 0	0 / 0	0 / 0	254 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.83 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-H, D-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 32	-84.3	-84.3 0.12 (1)	10.00	L-C	-248 / 79	0.05 (1)
B-C	-1281 / 0	-84.3	-84.3 0.16 (1)	5.49	C-K	0 / 1058	0.26 (1)
C-N	-1985 / 0	-84.3	-84.3 0.71 (1)	3.83	K-D	-138 / 144	0.04 (3)
N-O	-1985 / 0	-84.3	-84.3 0.71 (1)	3.83	D-I	-1572 / 0	0.46 (1)
O-P	-1985 / 0	-84.3	-84.3 0.71 (1)	3.83	I-E	0 / 154	0.04 (3)
P-D	-1985 / 0	-84.3	-84.3 0.71 (1)	3.83	I-F	0 / 1663	0.41 (1)
D-E	-744 / 0	-84.3	-84.3 0.49 (1)	6.14	E-L	0 / 1128	0.28 (1)
E-F	-620 / 0	-84.3	-84.3 0.13 (1)	6.25			
H-F	-1656 / 0	0.0	0.0 0.38 (1)	5.12			
M-B	-1329 / 0	0.0	0.0 0.15 (1)	6.99			
M-Q	0 / 0	-28.0	-28.0 0.10 (3)	10.00			
Q-L	0 / 0	-28.0	-28.0 0.10 (3)	10.00			
L-R	0 / 1055	-28.0	-28.0 0.25 (2)	10.00			
R-S	0 / 1055	-28.0	-28.0 0.25 (2)	10.00			
S-T	0 / 1055	-28.0	-28.0 0.25 (2)	10.00			
T-K	0 / 1055	-28.0	-28.0 0.25 (2)	10.00			
K-J	0 / 1989	-28.0	-28.0 0.47 (1)	10.00			
J-U	0 / 1989	-28.0	-28.0 0.47 (1)	10.00			
U-V	0 / 1989	-28.0	-28.0 0.47 (1)	10.00			
V-I	0 / 1989	-28.0	-28.0 0.47 (1)	10.00			
I-W	0 / 0	-28.0	-28.0 0.44 (1)	10.00			
W-H	0 / 0	-28.0	-28.0 0.44 (1)	10.00			
H-G	0 / 0	-28.0	-28.0 0.28 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
C	2-11-8	-6	-6	-	FRONT	VERT	DEAD
C	2-11-8	-45	-45	-	FRONT	VERT	SNOW
I	14-0-12	-147	-147	-	BACK	VERT	TOTAL
N	4-0-12	-6	-6	-	BACK	VERT	TOTAL
O	6-0-12	-6	-6	-	BACK	VERT	TOTAL
P	8-0-12	-17	-17	-	BACK	VERT	TOTAL
Q	2-0-12	-11	-11	-	BACK	VERT	TOTAL
R	4-0-12	-11	-11	-	BACK	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.57")
CALCULATED VERT. DEFL.(LL)= L/999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.57")
CALCULATED VERT. DEFL.(TL)= L/999 (0.19")

CSI: TC=0.71/1.00 (C-D:1), BC=0.47/1.00 (I-K:1), WB=0.46/1.00 (D-I:1), SSI=0.67/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
MT20	618	354	1667

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (I) (INPUT = 0.90)
JSI METAL= 0.62 (J) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286072	T245S	1	1	TRUSS DESC.	

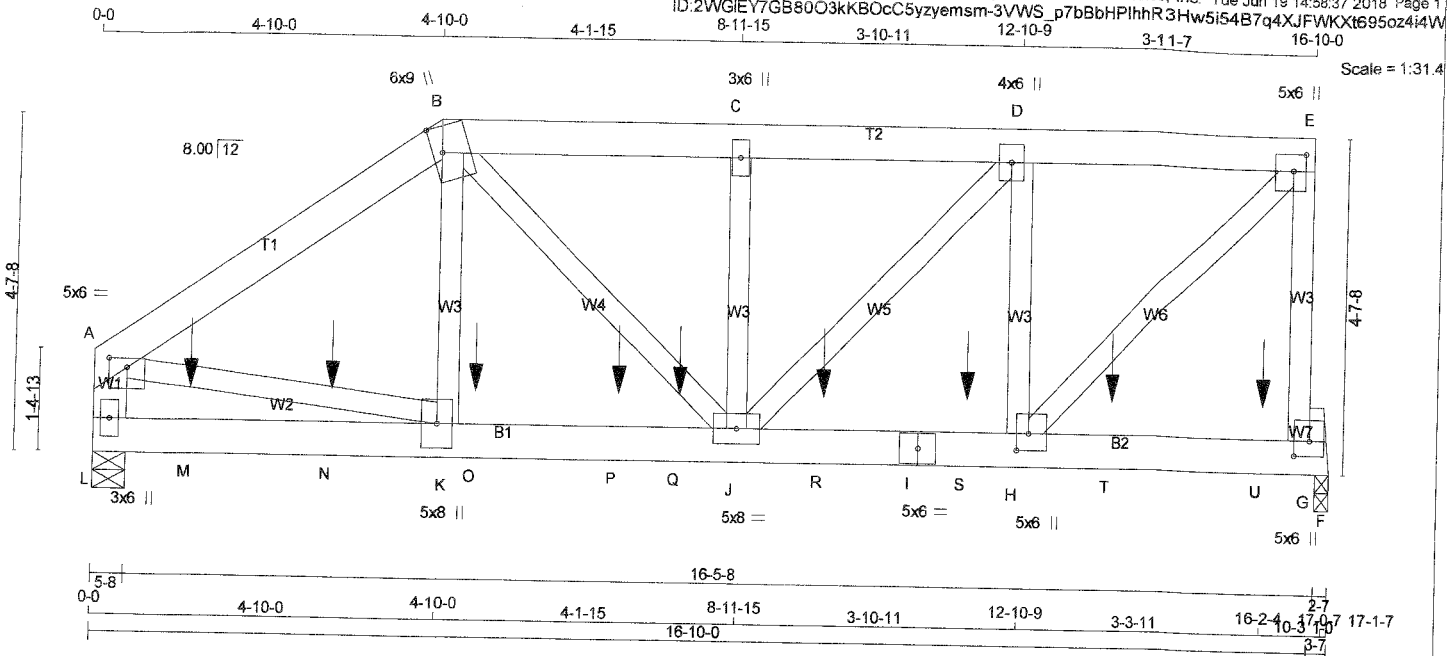
Tamarack Roof Truss, Burlington

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SPECIFIED CONCENTRATED LOADS (LBS)							
JT	LOC	LC1	MAX-	MAX+	FACE	DIR.	TYPE
S	6-0-12	-11	-11	---	BACK	VERT	TOTAL
T	8-0-12	-11	-11	---	BACK	VERT	TOTAL
U	10-0-12	-134	-134	---	BACK	VERT	TOTAL
V	12-0-12	-147	-147	---	BACK	VERT	TOTAL
W	16-0-12	-147	-147	---	BACK	VERT	TOTAL



T1800547-2



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - B	2x6	DRY	No.2	SPF
B - E	2x6	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
L - A	2x6	DRY	No.2	SPF
L - I	2x6	DRY	2100F 1.8E	SPF
I - F	2x6	DRY	2100F 1.8E	SPF
G - F	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 4 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
A-B	2	12
B-E	2	12
L-A	2	12
E-G	1	12
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
L-I	2	4
I-F	2	6
WEBS: (0.122"x3") SPIRAL NAILS		
2x4	1	5
J-B	1	5
C-J	1	6
J-D	1	6
D-H	1	6
H-E	1	6
K-A	1	6
2x6	2	6

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

IN ADDITION, PRE-DRILL ONE 0.56" DIAM. HOLE IN EACH CHORD PANEL AND INSTALL 0.50" DIAM. ASTM A307 BOLTS WITH WASHERS, BOTH SIDES. FOR OTHER BOLT TYPES SEE CSA086 3.3.2.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWV-p	MT20	5.0	6.0	1.50	3.00
B	TTWV+m	MT20	6.0	9.0	4.25	1.50
C	TMWV+w	MT20	3.0	6.0		
D	TMWV+t	MT20	4.0	6.0		
E	TMWV+p	MT20	5.0	6.0	2.75	2.00
G	BMWV+p	MT20	5.0	6.0	2.50	2.50
H	BMWV+t	MT20	5.0	6.0	2.75	2.00
I	BS-1	MT20	5.0	6.0		
J	BMWVW-t	MT20	5.0	8.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQ'D BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
L	8874	0	8874	0	0	5-8	5-8		
F	8889	0	8889	0	0	2-7	2-7		

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
	7184	4153 / 0	1527 / 0	0 / 0	0 / 0	1505 / 0	0 / 0
L	7231	4124 / 0	1574 / 0	0 / 0	0 / 0	1533 / 0	0 / 0

BEARING MATERIAL TO BE SPF 2100F 1.8E OR BETTER AT JOINT(S) L
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.94 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	MAX. FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	-10063 / 0	-84.3 -84.3	0.09 (1)	4.94	K-B	0 / 3830	0.15 (1)
B-C	-9831 / 0	-84.3 -84.3	0.08 (1)	5.00	B-J	0 / 1893	0.08 (1)
C-D	-9831 / 0	-84.3 -84.3	0.08 (1)	5.00	J-C	-281 / 9	0.01 (1)
D-E	-7360 / 0	-84.3 -84.3	0.06 (1)	5.61	J-D	0 / 3620	0.15 (1)
G-E	-7946 / 0	0.0 0.0	0.68 (1)	5.55	H-D	-2946 / 0	0.14 (1)
L-A	-7317 / 0	0.0 0.0	0.13 (1)	7.09	H-E	0 / 10693	0.43 (1)
					A-K	0 / 8489	0.34 (1)
L-M	0 / 0	-28.0 -28.0	0.25 (1)	10.00			
M-N	0 / 0	-28.0 -28.0	0.25 (1)	10.00			
N-K	0 / 0	-28.0 -28.0	0.25 (1)	10.00			
K-O	0 / 8489	-28.0 -28.0	0.36 (1)	10.00			
O-P	0 / 8489	-28.0 -28.0	0.36 (1)	10.00			
P-Q	0 / 8489	-28.0 -28.0	0.36 (1)	10.00			
Q-J	0 / 8489	-28.0 -28.0	0.36 (1)	10.00			
J-R	0 / 7360	-28.0 -28.0	0.31 (1)	10.00			
R-I	0 / 7360	-28.0 -28.0	0.31 (1)	10.00			
I-S	0 / 7360	-28.0 -28.0	0.31 (1)	10.00			
S-H	0 / 7360	-28.0 -28.0	0.31 (1)	10.00			
H-T	0 / 0	-28.0 -28.0	0.35 (1)	10.00			
T-U	0 / 0	-28.0 -28.0	0.35 (1)	10.00			
U-G	0 / 0	-28.0 -28.0	0.35 (1)	10.00			
G-F	0 / 0	-28.0 -28.0	0.20 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE
M	1-4-4	-1471	-1471	FRONT	VERT	TOTAL
N	3-4-4	-1471	-1471	FRONT	VERT	TOTAL
O	5-4-4	-1471	-1471	FRONT	VERT	TOTAL
P	7-4-4	-1471	-1471	FRONT	VERT	TOTAL
Q	8-2-4	-1394	-1394	FRONT	VERT	TOTAL
R	10-2-4	-1394	-1394	FRONT	VERT	TOTAL
S	12-2-4	-1394	-1394	FRONT	VERT	TOTAL
T	14-2-4	-1394	-1394	FRONT	VERT	TOTAL
U	16-2-4	-1395	-1395	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.57")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL) = L/360 (0.57")
CALCULATED VERT. DEFL.(TL) = L/999 (0.10")

CSI: TC=0.68/1.00 (E-G:1), BC=0.36/1.00 (J-K:1),
WB=0.43/1.00 (E-H:1), SSI=0.71/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE: GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (K) (INPUT = 0.90)
JSI METAL= 0.55 (K) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286072	T246	1	4	TRUSS DESC.	

Tamarack Roof Truss, Burlington

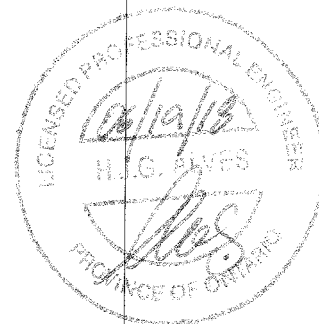
Version 8.210 S May 18 2018 MiTek Industries, Inc. Tue Jun 19 14:58:37 2018 Page 2
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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
K	BMWW+t	MT20	5.0	8.0		
L	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 1826.2 lbs FACTORED DOWN AT 1-4-4,
1826.2 lbs FACTORED DOWN AT 3-4-4, 1826.2
lbs FACTORED DOWN AT 5-4-4, 1826.2 lbs
FACTORED DOWN AT 7-4-4, 1711.9 lbs
FACTORED DOWN AT 8-2-4, 1711.9 lbs
FACTORED DOWN AT 10-2-4, 1711.9 lbs
FACTORED DOWN AT 12-2-4, AND 1711.9 lbs
FACTORED DOWN AT 14-2-4, AND 1712.9 lbs
FACTORED DOWN AT 16-2-4 ON BOTTOM
CHORD. DESIGN FOR UNSPECIFIED
CONNECTION(S) IS DELEGATED TO THE
BUILDING DESIGNER.



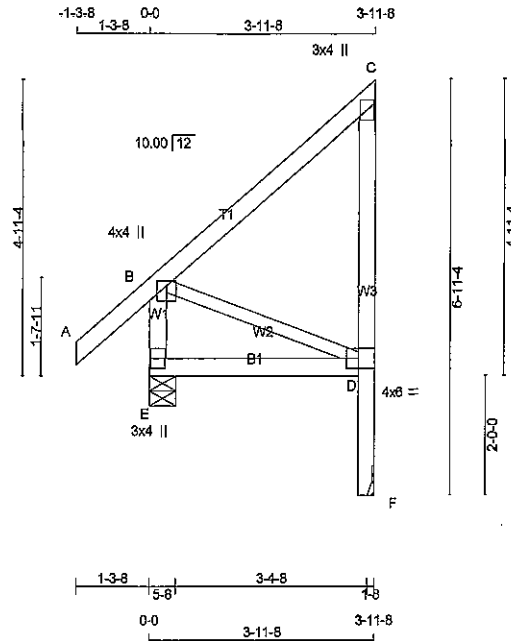
T1800546-2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
283746	T247	4	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:2WGIEY7GB8003kKBOcC5zyemsm-hk1gJ6I9VHMGG_FuALFq0W9Yg2Q7yDQU8NN5JQyehYh



Scale = 1:38.6

TOTAL WEIGHT = 4 X 24 = 96 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
F - C	2x4	DRY	No.2
E - B	2x4	DRY	No.2
E - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	4.0	4.0	1.00 2.00
C	TMV+p	MT20	3.0	4.0	
D	BMVW-l	MT20	4.0	6.0	2.00 2.50
E	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
F	222	0	222	0	0	5-8	5-8	5-8	5-8
E	339	0	339	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	182	101 / 0	42 / 0	0 / 0	0 / 0	40 / 0	0 / 0
E	282	172 / 0	42 / 0	0 / 0	0 / 0	48 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS		FACTORED		MAX. FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX	MAX	MEMB.	FORCE (LBS)	MAX	MAX
FR-TO		FROM	TO	LENGTH	FR-TO				
A-B	0 / 37	-84.3	-84.3	0.12 (1)	10.00	B-D	0 / 0	0.00 (1)	
B-C	0 / 0	-84.3	-84.3	0.22 (1)	10.00				
F-D	-222 / 0	0.0	0.0	0.03 (1)	7.81				
D-C	-167 / 0	0.0	0.0	0.08 (1)	7.81				
E-B	-283 / 0	0.0	0.0	0.03 (1)	7.81				
E-D	0 / 0	-28.0	-28.0	0.13 (3)	10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. GIC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.22 (B-C:1), BC=0.13 (D-E:3), WB=0.00 (B-D:1), SSI=0.10 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1867
	822	2284	1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)



DWG NO. TAM 46091-17
STRUCTURAL
COMPONENT ONLY

TOTAL WEIGHT = 150.00 (M)

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. G/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO
ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F.
RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED
ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.82")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.82")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.25 (G-H-1), BC=0.55 (M-O-1),
WB=0.43 (I-L-1), SS=0.23 (G-H-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN
THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 822 2284 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (H) (INPUT = 0.90)
JSI METAL= 0.62 (N) (INPUT = 1.00)

p/c

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
283746	T248	1	1	TRUSS DESC.		

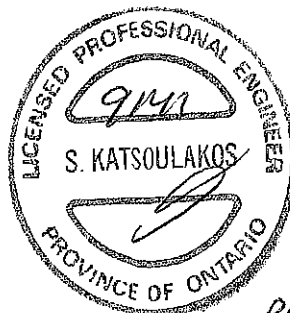
Tamarack Roof Truss, Burlington

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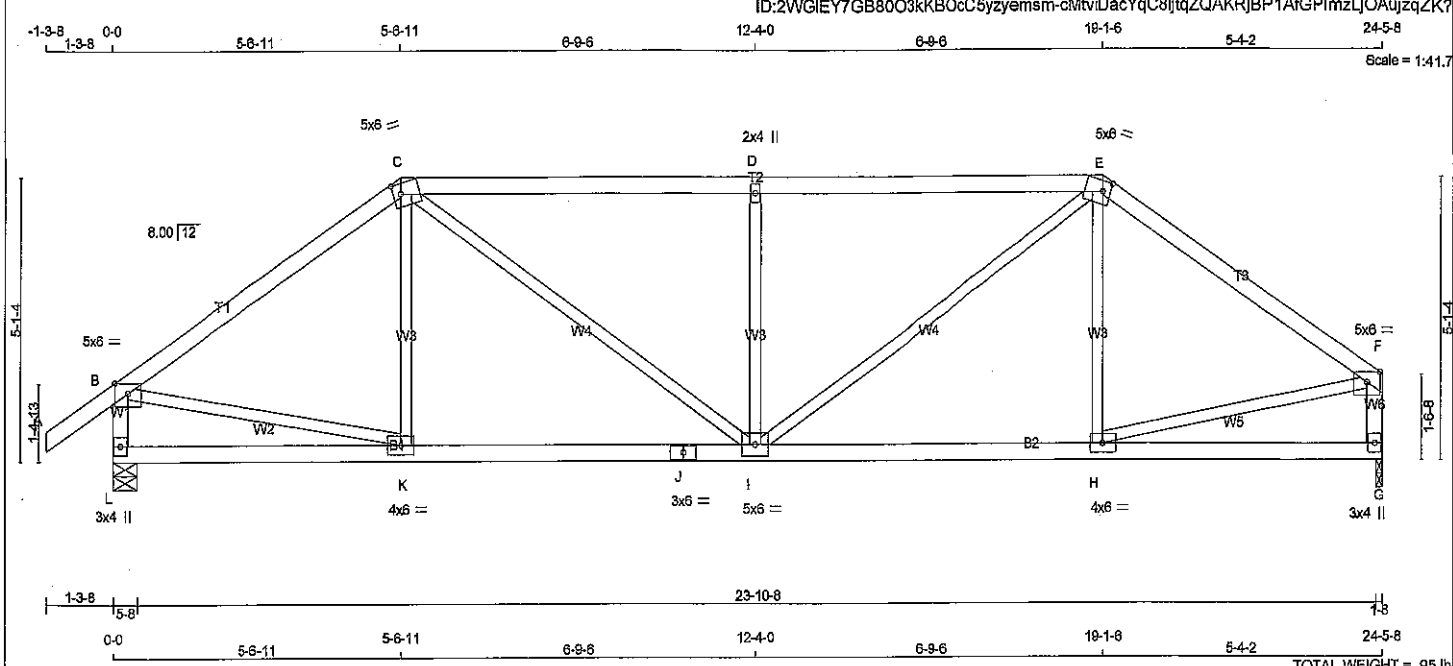
FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	4-0-11	-23	-25	—	FRONT	VERT	DEAD
C	4-0-11	-222	-222	—	FRONT	VERT	SNOW
H	20-7-8	-23	-25	—	FRONT	VERT	DEAD
H	20-7-8	-222	-222	—	FRONT	VERT	SNOW
S	5-0-12	-101	-101	—	BACK	VERT	TOTAL
T	7-0-12	-101	-101	—	BACK	VERT	TOTAL
U	9-0-12	-101	-101	—	BACK	VERT	TOTAL
V	11-0-12	-101	-101	—	BACK	VERT	TOTAL
W	13-0-12	-101	-101	—	BACK	VERT	TOTAL
X	15-0-12	-101	-101	—	BACK	VERT	TOTAL
Y	17-0-12	-101	-101	—	BACK	VERT	TOTAL
Z	19-0-12	-101	-101	—	BACK	VERT	TOTAL
AA	21-0-12	-125	-125	—	BACK	VERT	TOTAL
AB	1-0-12	-40	-70	—	BACK	VERT	TOTAL
AC	3-0-12	-40	-70	—	BACK	VERT	TOTAL
AD	5-0-12	-40	-70	—	BACK	VERT	TOTAL
AE	7-0-12	-40	-70	—	BACK	VERT	TOTAL
AF	9-0-12	-40	-70	—	BACK	VERT	TOTAL
AG	11-0-12	-40	-70	—	BACK	VERT	TOTAL
AH	13-0-12	-40	-70	—	BACK	VERT	TOTAL
AI	15-0-12	-40	-70	—	BACK	VERT	TOTAL
AJ	17-0-12	-40	-70	—	BACK	VERT	TOTAL
AK	19-0-12	-40	-70	—	BACK	VERT	TOTAL
AL	21-0-12	-40	-70	—	BACK	VERT	TOTAL
AM	23-0-12	-40	-70	—	BACK	VERT	TOTAL



DWG NO. TAM 46092-17
STRUCTURAL
COMPONENT ONLY

P64



TOTAL WEIGHT = 531

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
L - B	2x4	DRY	No.2
G - F	2x4	DRY	No.2
L - J	2x4	DRY	No.2
J - G	2x4	DRY	No.2

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B	TMWV-p	MT20	5.0	6.0	Edge
C	TTWW-m	MT20	5.0	6.0	2.25 1.75
D	TMWV-w	MT20	2.0	4.0	
E	TTWW-m	MT20	5.0	6.0	2.25 1.75
F	TMWV-p	MT20	5.0	6.0	Edge
G	BMV1+p	MT20	3.0	4.0	
H	BMWV-t	MT20	4.0	6.0	
I	BMWV-t	MT20	5.0	6.0	
J	BS-t	MT20	3.0	6.0	
K	BMWV-t	MT20	4.0	6.0	
L	BMV1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER							
BEARINGS							
	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG			
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
L	1488	0	1488	0	0	5-8	5-8
G	1373	0	1373	0	0	1-8	1-8

UNFACTORED REACTIONS							
JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1206	696 / 0	257 / 0	0 / 0	0 / 0	253 / 0	0 / 0
G	1127	628 / 0	257 / 0	0 / 0	0 / 0	245 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.81 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX UNBRACED LENGTH	MAX. FACTORED FORCE (LBS)
FR-TO		FROM	TO	FR-TO			
A-B	0 / 32	-84.3	-84.3 0.11 (1)	10.00	K-C	-46 / 211	0.05 (3)
B-C	-1508 / 0	-84.3	-84.3 0.55 (1)	4.88	C-I	0 / 744	0.17 (1)
C-D	-1859 / 0	-84.3	-84.3 0.83 (1)	3.81	I-D	-701 / 0	0.27 (1)
D-E	-1859 / 0	-84.3	-84.3 0.83 (1)	3.81	I-E	0 / 785	0.18 (1)
E-F	-1469 / 0	-84.3	-84.3 0.50 (1)	4.79	H-E	-83 / 188	0.04 (3)
L-B	-1426 / 0	0.0	0.0 0.15 (1)	6.83	B-K	0 / 1276	0.29 (1)
G-F	-1315 / 0	0.0	0.0 0.14 (1)	7.05	H-F	0 / 1250	0.28 (1)
L-K	0 / 0	-28.0	-28.0 0.25 (3)	10.00			
K-J	0 / 1252	-28.0	-28.0 0.40 (2)	10.00			
J-I	0 / 1252	-28.0	-28.0 0.40 (2)	10.00			
I-H	0 / 1219	-28.0	-28.0 0.39 (2)	10.00			
H-G	0 / 0	-28.0	-28.0 0.24 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:		
TOP CH.	LL =	25.6 PSF
	DL =	3.0 PSF
BOT CH.	LL =	10.5 PSF
	DL =	7.0 PSF
TOTAL LOAD	=	46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.82")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL)= L/260 (0.82")
CALCULATED VERT. DEFL.(TL) = L/999 (0.14")

CSI: TC=0.83/1.00 (C-D:1), BC=0.40/1.00 (H-K:2), WS=0.29/1.00 (B-K:1), SSI=0.28/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP(DRY)	SHEAR	SECTION		
	(PSI)	(PLI)	(PLI)		
	MAX MIN	MAX MIN	MAX MIN		
MT20	618 354	1667 622	2284 1656		

PLATE PLACEMENT TOL. = 0.250 inches

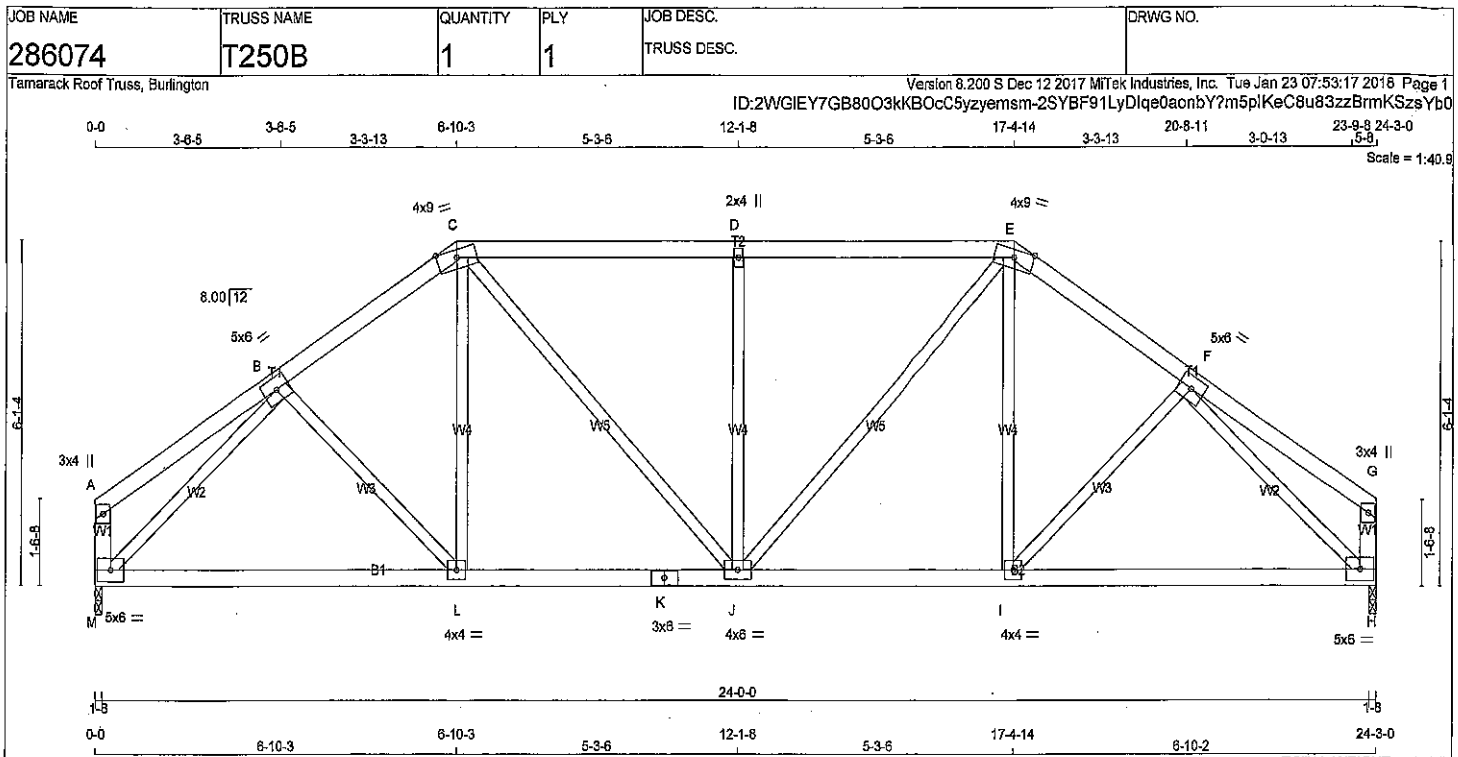
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (E) (INPUT = 0.90)
JSI METAL= 0.33 (J) (INPUT = 1.00)

DWG NO. TAM 6296-18
STRUCTURAL
COMPONENT ONLY



DRWG NO. TAM **6296-18**
STRUCTURAL COMPONENT ONLY



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4 DRY	No.2	SPF
C - E	2x4 DRY	No.2	SPF
E - G	2x4 DRY	No.2	SPF
M - A	2x4 DRY	No.2	SPF
H - G	2x4 DRY	No.2	SPF
M - K	2x4 DRY	No.2	SPF
K - H	2x4 DRY	No.2	SPF
ALL WEBS EXCEPT	2x3 DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	5.0	6.0		
C	TTWW-m	MT20	4.0	9.0	Edge	
D	TMV+w	MT20	2.0	4.0		
E	TTWW-m	MT20	4.0	9.0	Edge	
F	TMWW-t	MT20	5.0	6.0		
G	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	5.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMWW-t	MT20	4.0	6.0		
K	BS-t	MT20	3.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMVW1-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	UP	IN-SX
M	1361	0	0	1-8
H	1361	0	0	1-8

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN	COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERMLIVE	WIND	DEAD	SOIL
M	1118	620 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
H	1118	620 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.05 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 17	-84.3 -84.3	0.14 (1)	10.00	B-L	0 / 134	0.03 (3)
B-C	-1430 / 0	-84.3 -84.3	0.13 (1)	5.34	L-C	0 / 276	0.06 (2)
C-D	-1471 / 0	-84.3 -84.3	0.32 (1)	5.05	C-J	0 / 438	0.10 (1)
D-E	-1471 / 0	-84.3 -84.3	0.32 (1)	5.05	J-D	-543 / 0	0.32 (1)
E-F	-1430 / 0	-84.3 -84.3	0.13 (1)	5.34	J-E	0 / 438	0.10 (1)
F-G	0 / 17	-84.3 -84.3	0.14 (1)	10.00	I-E	0 / 276	0.06 (2)
M-A	-114 / 0	0.0 0.0	0.01 (1)	7.81	I-F	0 / 134	0.03 (3)
H-G	-114 / 0	0.0 0.0	0.01 (1)	7.81	M-B	-1633 / 0	0.62 (1)
					F-H	-1633 / 0	0.62 (1)
M-L	0 / 1141	-28.0 -28.0	0.43 (2)	10.00			
L-K	0 / 1176	-28.0 -28.0	0.44 (2)	10.00			
K-J	0 / 1176	-28.0 -28.0	0.44 (2)	10.00			
J-I	0 / 1176	-28.0 -28.0	0.44 (2)	10.00			
I-H	0 / 1141	-28.0 -28.0	0.43 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
DL =	3.0	PSF	
BQT CH.	LL =	10.5	PSF
DL =	7.0	PSF	
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L^3/360 (0.81")$
CALCULATED VERT. DEFL.(LL) = $L^3/999 (0.10")$
ALLOWABLE DEFL.(TL) = $L^3/360 (0.81")$
CALCULATED VERT. DEFL.(TL) = $L^3/999 (0.17")$

CSI: TC=0.32/1.00 (C-D:1), BC=0.44/1.00 (J-L:2), WB=0.62/1.00 (B-M:1), SSI=0.21/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.90 (E) (INPUT = 0.90)
JSI METAL= 0.40 (B) (INPUT = 1.00)



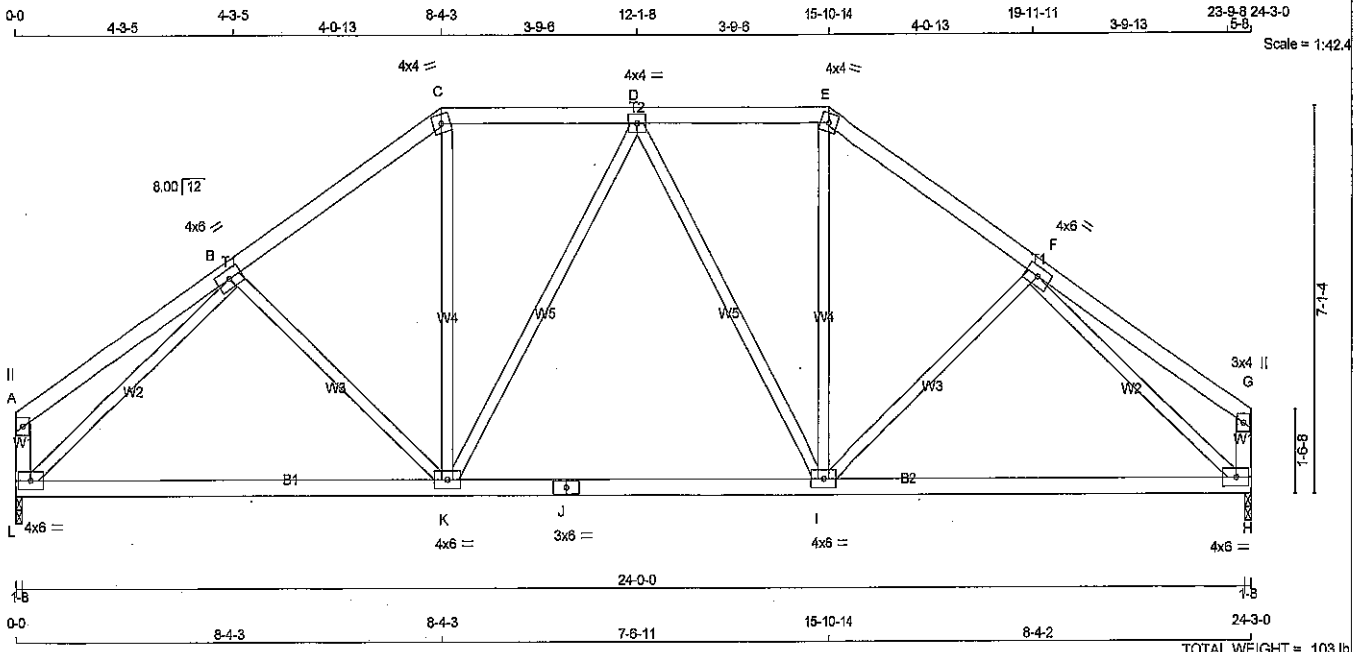
DRWG NO. TAM 5134
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286074	T251B	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
L - A	2x4	DRY	No.2	SPF	
H - G	2x4	DRY	No.2	SPF	
L - J	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	6.0		
C	TTW-m	MT20	4.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	6.0		
G	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
	VERT	HORZ	DOWN	HORZ
JT	1381	0	1381	0
L	1381	0	1381	0
H	1381	0	1381	0

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN.	COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
JT	1118	620 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0	
L	1118	620 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0	
H	1118	620 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.36 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 22	-84.3 -84.3	0.23 (1)	10.00	B-K	-77 / 99	0.05 (1)
B-C	-1368 / 0	-84.3 -84.3	0.20 (1)	5.36	K-C	0 / 465	0.11 (1)
C-D	-1126 / 0	-84.3 -84.3	0.16 (1)	5.82	D-I	-217 / 0	0.27 (1)
D-E	-1126 / 0	-84.3 -84.3	0.16 (1)	5.82	I-E	-217 / 0	0.27 (1)
E-F	-1368 / 0	-84.3 -84.3	0.23 (1)	10.00	F-H	-77 / 99	0.05 (1)
F-G	0 / 22	-84.3 -84.3	0.23 (1)	10.00	L-B	-1628 / 0	0.90 (1)
L-A	-136 / 0	0.0 0.0	0.01 (1)	7.81	F-H	-1628 / 0	0.90 (1)
H-G	-136 / 0	0.0 0.0	0.01 (1)	7.81			
L-K	0 / 1175	-28.0 -28.0	0.55 (2)	10.00			
K-J	0 / 1229	-28.0 -28.0	0.56 (2)	10.00			
J-I	0 / 1229	-28.0 -28.0	0.56 (2)	10.00			
I-H	0 / 1175	-28.0 -28.0	0.55 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL)= L/999 (0.21")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL)= L/843 (0.35")

CSI: TC=0.23/1.00 (A-B:1), BC=0.56/1.00 (I-K:2), WS=0.90/1.00 (B-L:1), SS=0.19/1.00 (K-L:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354 1667 822	2284 1658

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.40 (F) (INPUT = 1.00)



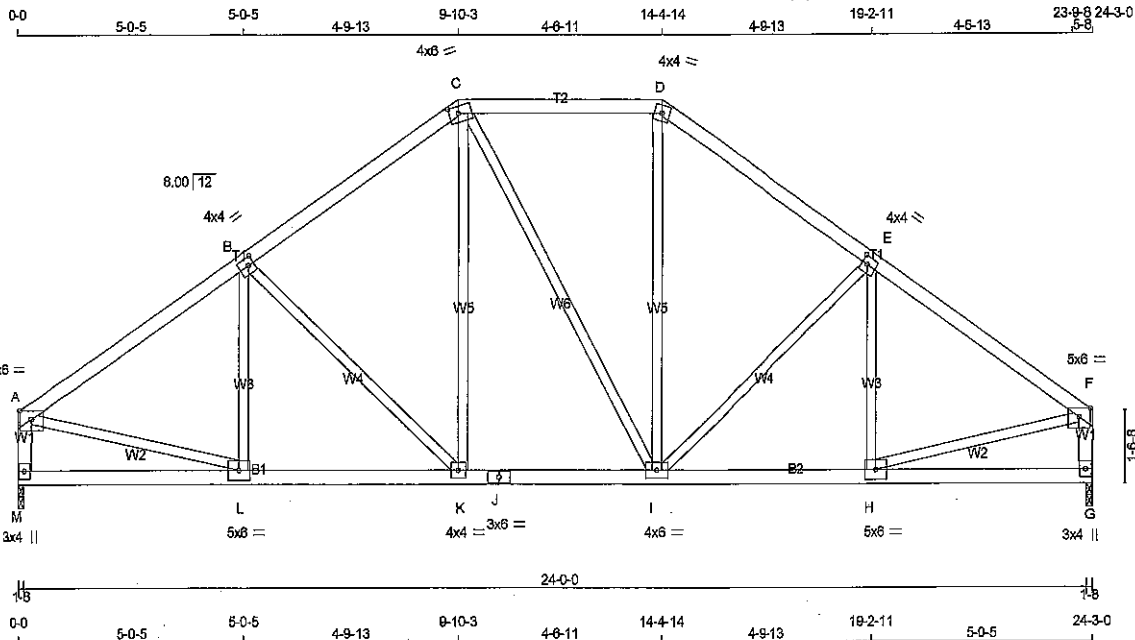
DWG NO. TAM 5/35 -18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286074	T252B	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:2WGIEY7GB80O3KKBOcC5zyemsm-Wf5ZTV2zjXQhG99_LJ3EJIMJa2ZRdfR6BrbJsuzsYp?



Scale = 1:48.8

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
M - A	2x4	DRY	No.2
G - F	2x4	DRY	No.2
M - J	2x4	DRY	No.2
J - G	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	6.0	Edge	
B	TMVW-t	MT20	4.0	4.0	2.00	1.50
C	TTWW-m	MT20	4.0	6.0	1.75	2.50
D	TTWW-m	MT20	4.0	4.0		
E	TMVW-t	MT20	4.0	4.0	2.00	1.50
F	TMVW-p	MT20	5.0	6.0	Edge	
G	BMV1+p	MT20	3.0	4.0		
H	BMVW-t	MT20	5.0	6.0		
I	BMVW-t	MT20	4.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMVW-t	MT20	4.0	4.0		
L	BMVW-t	MT20	5.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
JT	VERT	HORZ	GROSS REACTION	DOWN	HORZ	BRG	IN-SX	BRG	IN-SX
M	1351	0	1351	0	0	1-8	1-8	1-8	1-8
G	1351	0	1351	0	0	1-8	1-8	1-8	1-8

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
M	1118	620 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
G	1118	620 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.09 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	MAX. FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FORCE (LBS)	MAX. CSI (LC)	MAX. CSI (LC)
FR-TO		FROM	TO	FR-TO			
A-B	-1480 / 0	-84.3	-84.3 0.29 (1)	5.09	L-B	-148 / 125	0.05 (1)
B-C	-1259 / 0	-84.3	-84.3 0.28 (1)	5.43	B-K	-315 / 0	0.26 (1)
C-D	-1028 / 0	-84.3	-84.3 0.24 (1)	5.91	K-C	0 / 367	0.08 (2)
D-E	-1260 / 0	-84.3	-84.3 0.28 (1)	5.42	C-I	0 / 2	0.00 (2)
E-F	-1479 / 0	-84.3	-84.3 0.29 (1)	5.09	I-D	0 / 368	0.08 (2)
M-A	-1303 / 0	0.0	0.0 0.14 (1)	7.08	E-H	-314 / 0	0.26 (1)
G-F	-1303 / 0	0.0	0.0 0.14 (1)	7.08	H-E	-150 / 124	0.05 (1)
					A-L	0 / 1280	0.23 (1)
					H-F	0 / 1289	0.29 (1)
M-L	0 / 0	-28.0	-28.0 0.17 (3)	10.00			
L-K	0 / 1253	-28.0	-28.0 0.31 (2)	10.00			
K-J	0 / 1027	-28.0	-28.0 0.23 (2)	10.00			
J-I	0 / 1027	-28.0	-28.0 0.23 (2)	10.00			
I-H	0 / 1253	-28.0	-28.0 0.31 (2)	10.00			
H-G	0 / 0	-28.0	-28.0 0.17 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
DL = 3.0	PSF	
BOT CH.	LL = 10.5	PSF
DL = 7.0	PSF	
TOTAL LOAD = 46.1	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/380 (0.81")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/999 (0.08")

CSI: TC=0.29/1.00 (A-B:1), BC=0.31/1.00 (K-L:2),
WB=0.29/1.00 (A-L:1), SS=0.17/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (A) (INPUT = 0.90)
JSI METAL= 0.31 (L) (INPUT = 1.00)



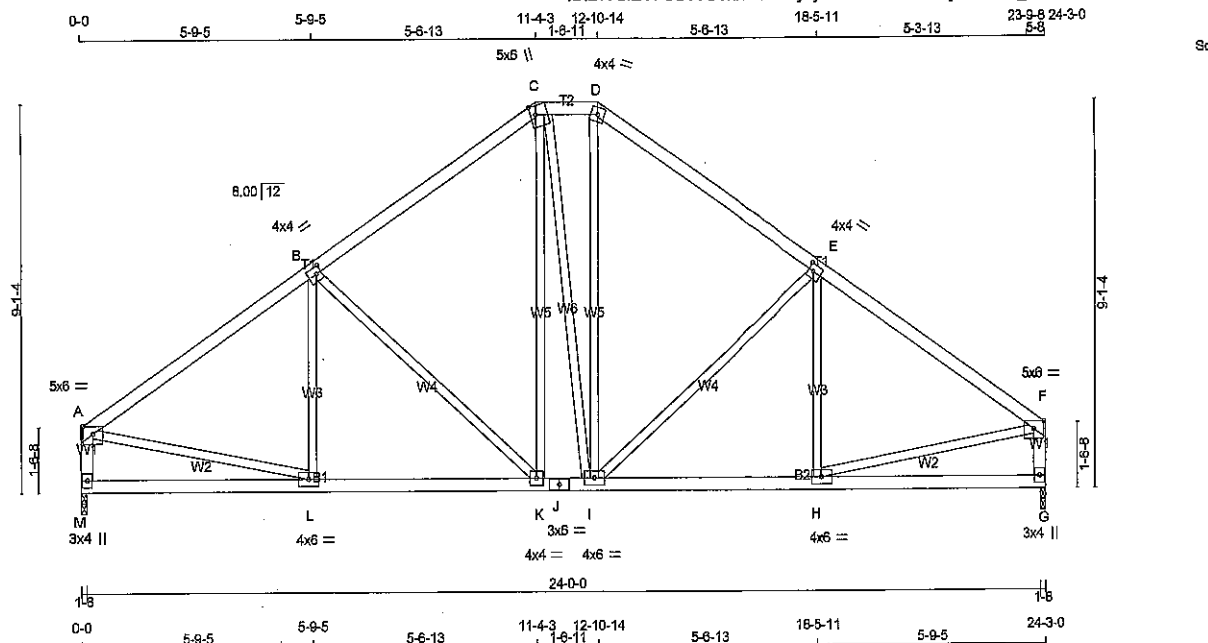
DRWG NO. TAM 5136 -B
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286074	T253B	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 113 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
M - A	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
M - J	2x4	DRY	No.2	SPF	
J - G	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A - TMWW-p	MT20	5.0	6.0	Edge	
B - TMWW-t	MT20	4.0	4.0	2.00	1.50
C - TTWW+m	MT20	5.0	6.0	2.50	1.50
D - TTW-m	MT20	4.0	4.0		
E - TMWW-t	MT20	4.0	4.0	2.00	1.50
F - TMWW-p	MT20	5.0	6.0	Edge	
G - BMV1+p	MT20	3.0	4.0		
H - BMWW-t	MT20	4.0	6.0		
I - BMWW-t	MT20	4.0	6.0		
J - BS-t	MT20	3.0	6.0		
K - BMWW-t	MT20	4.0	4.0		
L - BMWW-t	MT20	4.0	6.0		
M - BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQ'D
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	UPLIFT	IN-SX
M	1381	0	1381	0
G	1381	0	1381	0

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1118	820 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
G	1118	820 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.94 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)	UNBRACED LENGTH
FR-TO				FR-TO			
A-B	-1488 / 0	-84.3	-84.3 0.40 (1)	4.94	L-B	-81 / 195	0.04 (3)
B-C	-1156 / 0	-84.3	-84.3 0.37 (1)	5.47	B-K	-443 / 0	0.52 (1)
C-D	-941 / 0	-84.3	-84.3 0.03 (1)	6.25	K-C	0 / 366	0.08 (1)
D-E	-1180 / 0	-84.3	-84.3 0.37 (1)	5.46	C-I	0 / 16	0.00 (1)
E-F	-1486 / 0	-84.3	-84.3 0.40 (1)	4.94	I-D	0 / 385	0.09 (1)
M-A	-1296 / 0	0.0	0.0 0.14 (1)	7.09	I-E	-438 / 0	0.51 (1)
G-F	-1295 / 0	0.0	0.0 0.14 (1)	7.10	H-E	-87 / 190	0.04 (3)
					A-L	0 / 1292	0.29 (1)
					H-F	0 / 1291	0.29 (1)
M-L	0 / 0	-28.0	-28.0 0.24 (3)	10.00			
L-K	0 / 1264	-28.0	-28.0 0.38 (2)	10.00			
K-J	0 / 938	-28.0	-28.0 0.24 (2)	10.00			
J-I	0 / 938	-28.0	-28.0 0.24 (2)	10.00			
I-H	0 / 1263	-28.0	-28.0 0.37 (2)	10.00			
H-G	0 / 0	-28.0	-28.0 0.23 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 25.6	PSF
	DL	= 3.0	PSF
BOT CH.	LL	= 10.5	PSF
	DL	= 7.0	PSF
TOTAL LOAD		= 46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 085-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL)= L/999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL)= L/999 (0.10")

CSI: TC=0.40/1.00 (A-B:1), BC=0.38/1.00 (K-L:2),
WB=0.52/1.00 (B-K:1), SSI=0.19/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 622 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (L) (INPUT = 0.90)
JSI METAL= 0.33 (J) (INPUT = 1.00)



DRWG NO. TAM 5137-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286074	T254B	3	1	TRUSS DESC.	

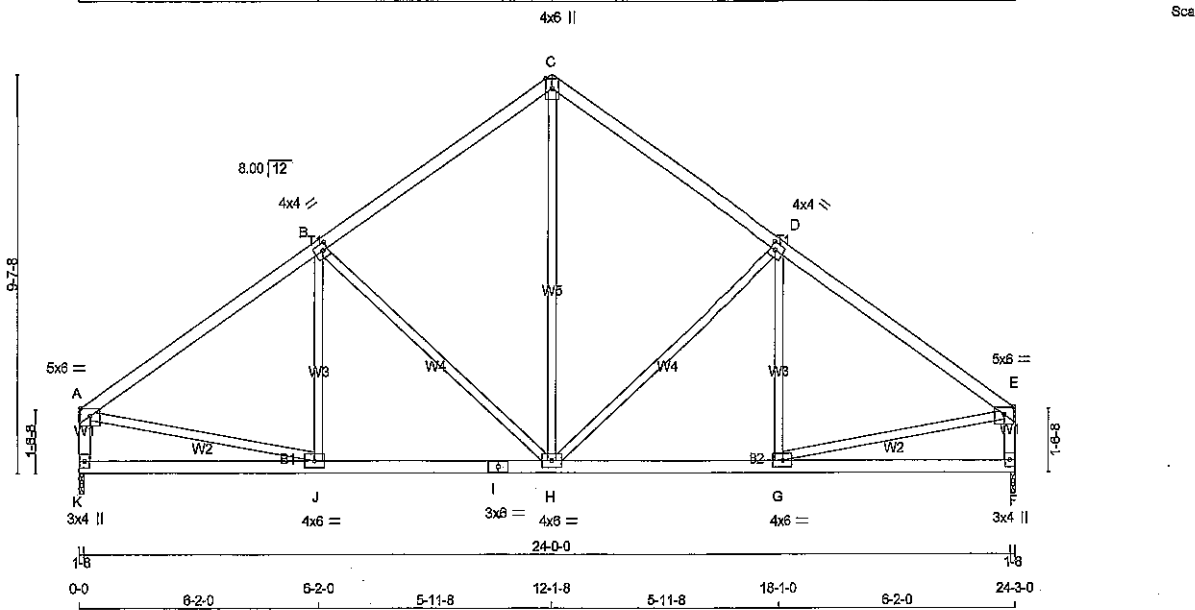
Tamarack Roof Truss, Burlington

Version 8.200 S Dec 12 2017 MiTek Industries, Inc. Tue Jan 23 07:53:19 2018 Page 1

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0-0 8-2-0 6-2-0 5-11-8 12-1-8 5-11-8 18-1-0 5-8-8 23-9-8 24-3-0

Scale = 1:55.9



TOTAL WEIGHT = 3 X 101 = 304 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4 DRY	No.2	SPF		
C - E	2x4 DRY	No.2	SPF		
K - A	2x4 DRY	No.2	SPF		
F - E	2x4 DRY	No.2	SPF		
K - I	2x4 DRY	No.2	SPF		
I - F	2x4 DRY	No.2	SPF		
ALL WEBS	2x3 DRY	No.2	SPF		
EXCEPT					

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	6.0	Edge	
B	TMVW-t	MT20	4.0	4.0	2.00	1.50
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMVW-t	MT20	4.0	4.0	2.00	1.50
E	TMVW-p	MT20	5.0	6.0	Edge	
F	BMV1+p	MT20	3.0	4.0		
G	BMVW-t	MT20	4.0	6.0		
H	BMVW-t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMVW-t	MT20	4.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	UPLIFT
K	1361	0	1361	0
F	1361	0	1361	0

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
K	1118	620 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0	0 / 0
F	1118	620 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.86 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO				FR-TO			
A-B	-1484 / 0	-84.3	-84.3 0.46 (1)	4.86	H-C	0 / 817	0.18 (1)
B-C	-1110 / 0	-84.3	-84.3 0.42 (1)	5.47	H-D	-494 / 0	0.66 (1)
C-D	-1110 / 0	-84.3	-84.3 0.42 (1)	5.47	G-D	-80 / 214	0.05 (3)
D-E	-1484 / 0	-84.3	-84.3 0.46 (1)	4.86	B-H	-494 / 0	0.66 (1)
K-A	-1291 / 0	0.0	0.0 0.14 (1)	7.10	J-B	-80 / 214	0.05 (3)
F-E	-1291 / 0	0.0	0.0 0.14 (1)	7.10	A-J	0 / 1267	0.29 (1)
					G-E	0 / 1267	0.29 (1)
K-J	0 / 0	-28.0	-28.0 0.26 (3)	10.00			
J-I	0 / 1263	-28.0	-28.0 0.40 (2)	10.00			
I-H	0 / 1263	-28.0	-28.0 0.40 (2)	10.00			
H-G	0 / 1263	-28.0	-28.0 0.40 (2)	10.00			
G-F	0 / 0	-28.0	-28.0 0.26 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 25.6 PSF
	DL	= 3.0 PSF
BOT CH.	LL	= 10.5 PSF
	DL	= 7.0 PSF
TOTAL LOAD		= 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TRC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.61")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.07")
ALLOWABLE DEFL.(TL) = $L/360$ (0.61")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.11")

CSI: TC=0.46/1.00 (A-B:1), BC=0.40/1.00 (H-J:2), WB=0.66/1.00 (B-H:1), SS=0.21/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP (DRY)	SHEAR (PL)	SECTION (PL)
	MAX	MIN	MAX
MT20	618	354	1667

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (J) (INPUT = 0.80)
JSI METAL= 0.34 (I) (INPUT = 1.00)



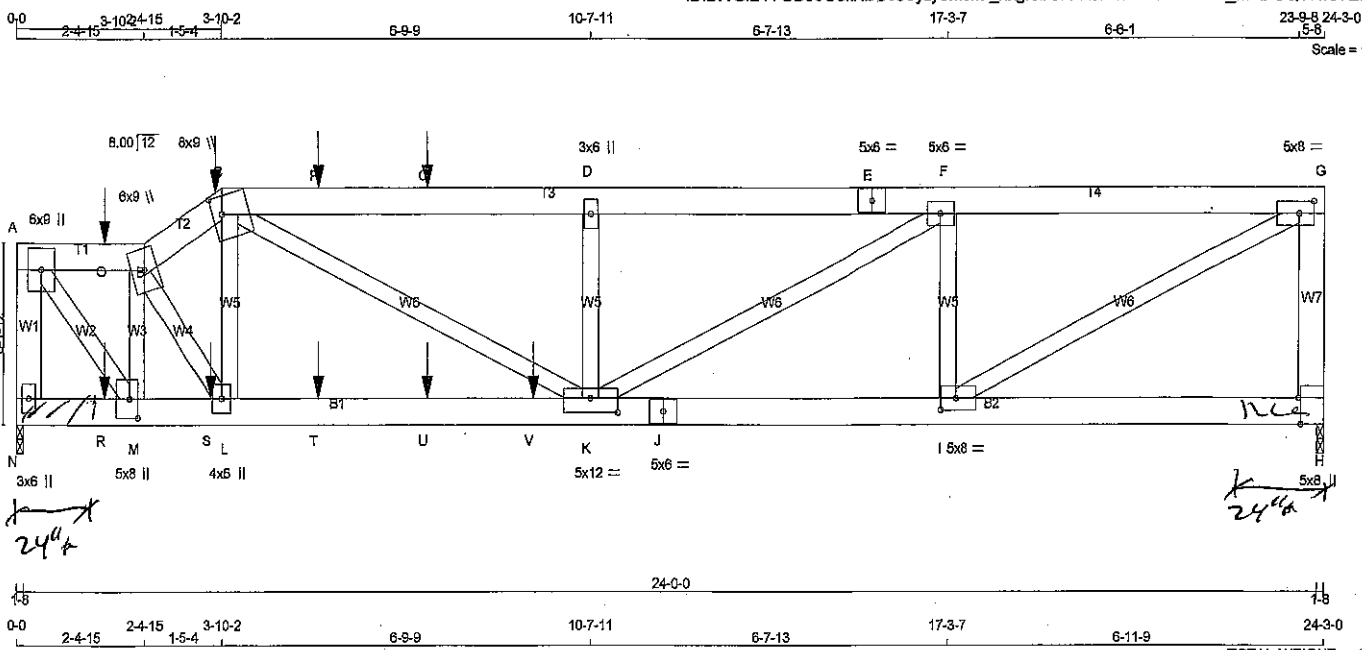
DRWG NO. TAM 513B-18
STRUCTURAL
COMPONENT ONLY

JOB NAME 286074	TRUSS NAME T255	QUANTITY 1	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:2WGIEY7GB8003kKBOC5zyemsm-rfgr3bUrYtJkAv0aTrWuazRs_M0DGQVKtOKzYb



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
N - A	2x6	DRY	No.2 SPF
A - B	2x6	DRY	No.2 SPF
B - C	2x6	DRY	No.2 SPF
C - E	2x6	DRY	No.2 SPF
E - G	2x6	DRY	No.2 SPF
H - G	2x6	DRY	No.2 SPF
N - J	2x6	DRY	2100F 1.8E SPF
J - H	2x6	DRY	2100F 1.8E SPF
ALL WEBS 2x4 DRY No.2 SPF			
DRY: SEASONED LUMBER.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	UP/LIFT
N	2881	0	2881	0
H	2026	0	2026	0

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
N	2335	1344 / 0	499 / 0	0 / 0	0 / 0	493 / 0
H	1655	932 / 0	398 / 0	0 / 0	0 / 0	355 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.31 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED HORZ. LOAD (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORZ. LOAD (LC)	
FR-TO		FROM TO		FR-TO			
N-A	-2739 / 0	0.0	0.0 0.26 (1)	A-M	0 / 3416	0.60 (1)	
A-O	-2200 / 0	-84.3	-84.3 0.12 (1)	M-B	-3004 / 0	0.39 (1)	
O-B	-2200 / 0	-84.3	-84.3 0.12 (1)	B-L	0 / 951	0.17 (1)	
B-C	-3425 / 0	-84.3	-84.3 0.07 (1)	L-C	0 / 315	0.06 (3)	
C-P	-4927 / 0	-84.3	-84.3 0.64 (1)	I-G	0 / 3520	0.64 (1)	
P-Q	-4927 / 0	-84.3	-84.3 0.64 (1)	C-K	0 / 2353	0.42 (1)	
Q-D	-4927 / 0	-84.3	-84.3 0.64 (1)	I-F	-1559 / 0	0.27 (1)	
D-E	-4927 / 0	-84.3	-84.3 0.48 (1)	K-D	-692 / 0	0.12 (1)	
E-F	-4927 / 0	-84.3	-84.3 0.48 (1)	K-F	0 / 1961	0.35 (1)	
F-G	-3207 / 0	-84.3	-84.3 0.35 (1)				
H-G	-1925 / 0	0.0	0.0 0.26 (1)				
N-R	0 / 0	-28.0	-28.0 0.08 (1)				
R-M	0 / 0	-28.0	-28.0 0.08 (1)				
M-S	0 / 2364	-28.0	-28.0 0.34 (1)				
S-L	0 / 2364	-28.0	-28.0 0.34 (1)				
L-T	0 / 2851	-28.0	-28.0 0.48 (1)				
T-U	0 / 2851	-28.0	-28.0 0.48 (1)				
U-V	0 / 2851	-28.0	-28.0 0.48 (1)				
V-K	0 / 2851	-28.0	-28.0 0.48 (1)				
K-J	0 / 3207	-28.0	-28.0 0.33 (1)				
J-I	0 / 3207	-28.0	-28.0 0.33 (1)				
I-H	0 / 0	-28.0	-28.0 0.10 (2)				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE
C	3-10-2	-104	-104	FRONT	VERT	TOTAL
C	3-10-2	-21	-24	FRONT	VERT	DEAD
C	3-10-2	-212	-212	FRONT	VERT	SNOW
O	1-7-12	-145	-145	FRONT	VERT	TOTAL
P	5-7-12	-101	-101	FRONT	VERT	TOTAL
Q	7-7-12	-101	-101	FRONT	VERT	TOTAL
R	1-7-12	-83	-90	FRONT	VERT	TOTAL
S	3-7-12	-40	-70	FRONT	VERT	TOTAL
T	5-7-12	-40	-70	FRONT	VERT	TOTAL
U	7-7-12	-40	-70	FRONT	VERT	TOTAL
V	9-7-0	-1298	-1298	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL	LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2012, 8CBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/999 (0.19")
ALLOWABLE DEFL.(TL) = L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/930 (0.31")

CSI: TO=0.64/1.00 (C-D:1), BC=0.48/1.00 (K-L:1), WB=0.64/1.00 (G-I:1), SSI=0.80/1.00 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MT20	618	354	1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (I) (INPUT = 0.90)
JSI METAL= 0.65 (J) (INPUT = 1.00)

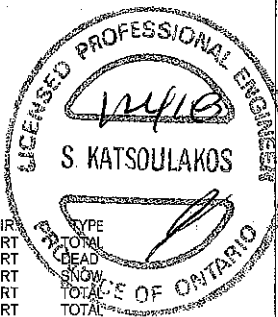
PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
A	TMWW+p	MT20	6.0	9.0
B	TTWW+m	MT20	6.0	9.0
C	TTWW+m	MT20	8.0	9.0 3.75 2.00
D	TMWW+w	MT20	3.0	6.0
E	TS-I	MT20	5.0	6.0
F	TMWW-I	MT20	5.0	6.0
G	TMWW-I	MT20	5.0	8.0 2.50 3.25
H	BMV1+p	MT20	5.0	8.0 Edge 0.50
I	BMWW-I	MT20	5.0	8.0 2.50 3.25
J	BS-I	MT20	5.0	8.0
K	BMWWW-I	MT20	5.0	12.0 3.00 6.00
L	BMWWW-I	MT20	4.0	6.0
M	BMWWW-I	MT20	5.0	8.0 4.00 2.00
N	BMV1+p	MT20	3.0	6.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 144.9 lbs FACTORED DOWN AT 1-7-12, 337.2 lbs FACTORED DOWN AT 3-10-2, AND 101.4 lbs FACTORED DOWN AT 5-7-12, AND 101.4 lbs FACTORED DOWN AT 7-7-12 ON TOP CHORD, AND 89.8 lbs FACTORED DOWN AT 1-7-12, 69.9 lbs FACTORED DOWN AT 3-7-12, 69.9 lbs FACTORED DOWN AT 5-7-12, AND 69.9 lbs FACTORED DOWN AT 7-7-12, AND 1297.6 lbs FACTORED DOWN AT 9-7-0 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

SCAB JOINT #N 24" WITH 2x6 #2
SPF 24" LONG 1 SIDE(S) USING
ROW(S) 24" ARDOX SPIRAL COMMON WIRE
NAILS AT 2" O/C STAGGERED 2 NAILS/SCAB.
REQUIRED FOR 1/2 BRG PROVISION.



DRWG NO. TAM 5137-08
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286074	T256	1	1	TRUSS DESC.	

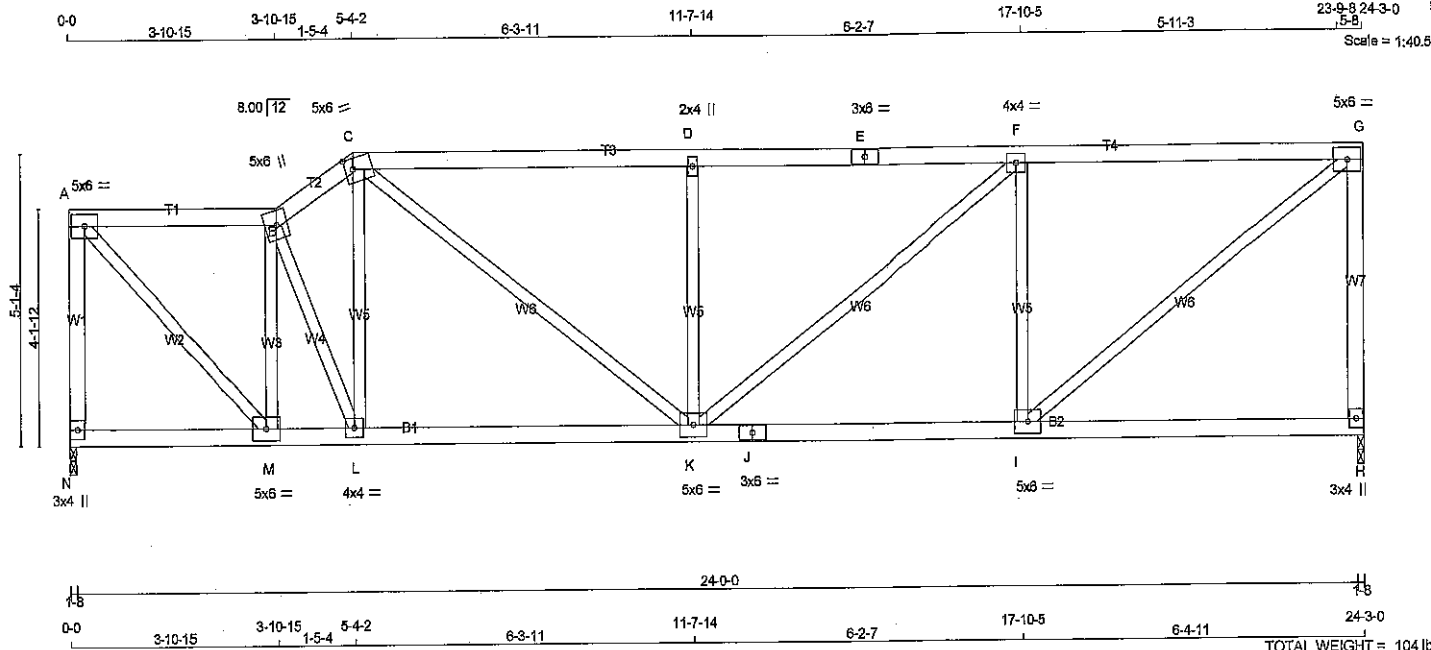
Tamarack Roof Truss, Burlington

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23-9-8 24-3-0

Scale = 1:40.5



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
N - A	2x4	DRY	No.2 SPF
A - B	2x4	DRY	No.2 SPF
B - C	2x4	DRY	No.2 SPF
C - E	2x4	DRY	No.2 SPF
E - G	2x4	DRY	No.2 SPF
H - G	2x4	DRY	No.2 SPF
N - J	2x4	DRY	No.2 SPF
J - H	2x4	DRY	No.2 SPF
ALL WEBS	2x3	DRY	No.2 SPF
DRY: SEASONED LUMBER.			

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
A	TMVW-t	MT20	5.0	6.0
B	TTWW+m	MT20	5.0	6.0
C	TTWW-m	MT20	5.0	6.0
D	TMVW-w	MT20	2.0	4.0
E	TS-t	MT20	3.0	6.0
F	TMVW-t	MT20	4.0	4.0
G	TMVW-t	MT20	5.0	6.0
H	BMV1+p	MT20	3.0	4.0
I	BMVW-t	MT20	5.0	6.0
J	BS-t	MT20	3.0	6.0
K	BMVW-t	MT20	5.0	6.0
L	BMVW-t	MT20	4.0	4.0
M	BMVW-t	MT20	5.0	6.0
N	BMV1+p	MT20	3.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
		GROSS REACTION		GROSS REACTION		BRG		BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
N	1361	0	1361	0	0	1-8	1-8	1-8	1-8
H	1361	0	1361	0	0	1-8	1-8	1-8	1-8

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN, COMPONENT REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
N	1118	620 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0		
H	1118	620 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.33 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX. FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX	MAX. UNBRAC	MEMB.	FORCE (LBS)	MAX	MAX. CSI (LC)
FR-TO		FROM TO				FR-TO			
N-A	-1312 / 0	0.0	0.0	0.35 (1)	7.05	A-M	0 / 1618	0.36 (1)	
A-B	-1136 / 0	-84.3	-84.3	0.24 (1)	5.66	M-B	-1103 / 0	0.29 (1)	
B-C	-1438 / 0	-84.3	-84.3	0.05 (1)	5.41	B-L	0 / 81	0.02 (1)	
C-D	-1783 / 0	-84.3	-84.3	0.58 (1)	4.36	L-C	0 / 177	0.04 (3)	
D-E	-1783 / 0	-84.3	-84.3	0.57 (1)	4.33	I-G	0 / 1779	0.40 (1)	
E-F	-1783 / 0	-84.3	-84.3	0.57 (1)	4.33	C-K	0 / 748	0.17 (1)	
F-G	-1422 / 0	-84.3	-84.3	0.54 (1)	4.79	I-F	-672 / 0	0.34 (1)	
H-G	-1288 / 0	0.0	0.0	0.57 (1)	7.11	K-D	-571 / 0	0.22 (1)	
						K-F	0 / 470	0.11 (1)	
N-M	0 / 0	-28.0	-28.0	0.10 (2)	10.00				
M-L	0 / 1186	-28.0	-28.0	0.30 (2)	10.00				
L-K	0 / 1186	-28.0	-28.0	0.34 (2)	10.00				
K-J	0 / 1422	-28.0	-28.0	0.44 (2)	10.00				
J-I	0 / 1422	-28.0	-28.0	0.44 (2)	10.00				
I-H	0 / 0	-28.0	-28.0	0.27 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/999 (0.09")
ALLOWABLE DEFL.(TL) = L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/999 (0.16")

CSI: TC=0.57/1.00 (D-F:1), BC=0.44/1.00 (I-K:2), WB=0.40/1.00 (G-I:1), SSI=0.25/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.65 (G) (INPUT = 0.90)
JSI METAL= 0.36 (J) (INPUT = 1.00)

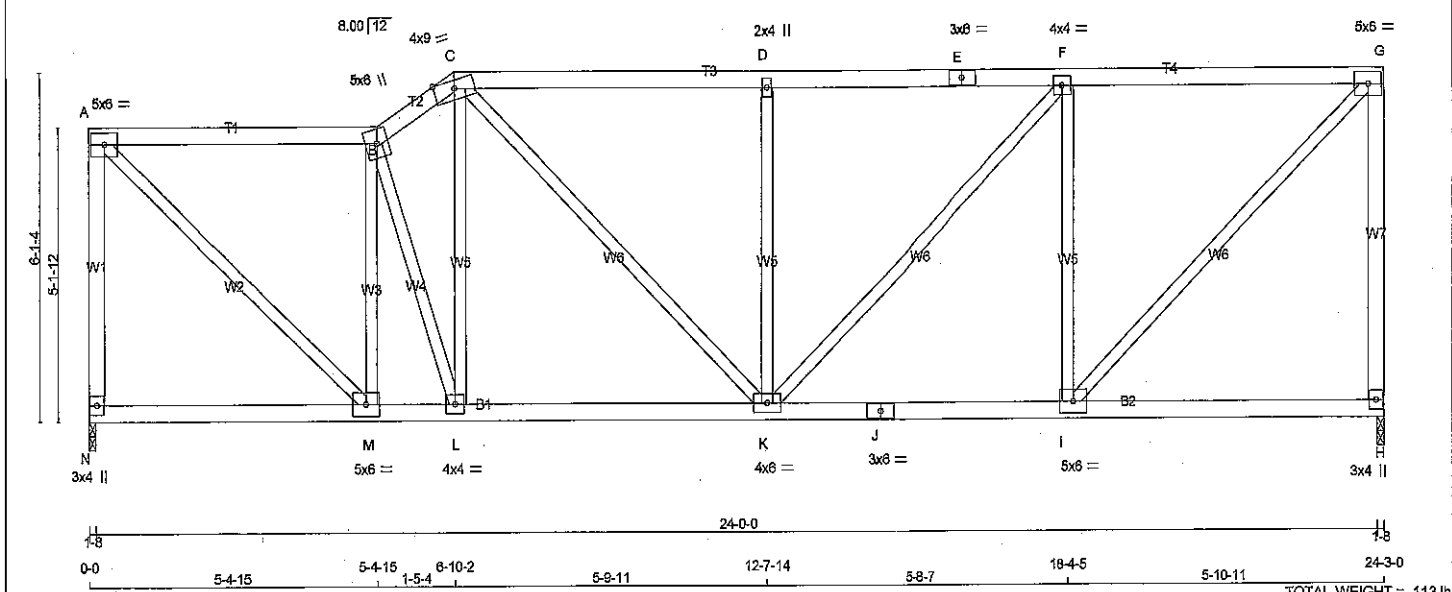


DRWG NO. TAM 5140-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286074	T257	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

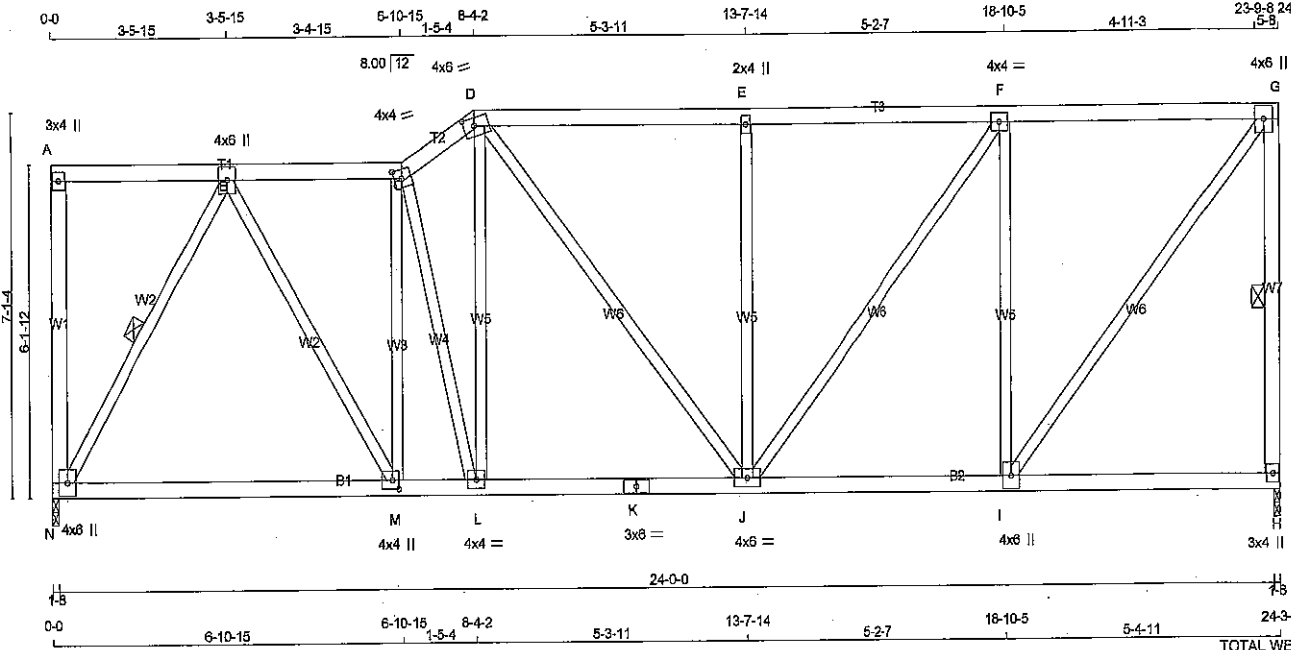
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23-9-8 24-3-0
Scale = 1:40.5



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. N - A 2x4 DRY No.2 SPF A - B 2x4 DRY No.2 SPF B - C 2x4 DRY No.2 SPF C - E 2x4 DRY No.2 SPF E - G 2x4 DRY No.2 SPF H - G 2x4 DRY No.2 SPF N - J 2x4 DRY No.2 SPF J - H 2x4 DRY No.2 SPF ALL WEBS 2x3 DRY No.2 SPF DRY: SEASONED LUMBER.				DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 25.6 PSF DL = 3.0 PSF BOT CH. LL = 10.5 PSF DL = 7.0 PSF TOTAL LOAD = 46.1 PSF SPACING = 24.0 IN. C/C LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12 THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010 THIS DESIGN COMPLIES WITH: - PART 9 OF CBC 2012, BCBC 2012, ABC 2014 - CSA 086-09 - TPIC 2011 (55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL)= L/360 (0.61") CALCULATED VERT. DEFL.(LL) = L/999 (0.08") ALLOWABLE DEFL.(TL)= L/360 (0.81") CALCULATED VERT. DEFL.(TL) = L/999 (0.14") CSI: TC=0.93/1.00 (G-H:1), BC=0.36/1.00 (I-K:2), WS=0.53/1.00 (F-I:1), SSI=0.23/1.00 (F-G:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10 COMPANION LIVE LOAD FACTOR = 0.50 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 618 354 1667 822 2284 1656 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. SI GRIP= 0.86 (C) (INPUT = 0.90) SI METAL= 0.34 (J) (INPUT = 1.00)				PLATES (table is in inches) JT TYPE PLATES W LEN Y X A TMWW-t MT20 5.0 6.0 B TTWW+m MT20 5.0 6.0 C TTWW-m MT20 4.0 9.0 Edge D TMWW+w MT20 2.0 4.0 E TS-t MT20 3.0 6.0 F TMWW-t MT20 4.0 4.0 G TMWW-t MT20 5.0 6.0 H BMV1+p MT20 3.0 4.0 I BMWW-t MT20 5.0 6.0 J BS-t MT20 3.0 6.0 K BMWW-t MT20 4.0 6.0 L BMWW-t MT20 4.0 4.0 M BMWW-t MT20 5.0 6.0 N BMV1+p MT20 3.0 4.0 Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.				BEARINGS FACTORED MAXIMUM FACTORED INPUT REQD GROSS REACTION GROSS REACTION BRG BRG JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX N 1361 0 1361 0 0 1-8 1-8 H 1361 0 1361 0 0 1-8 1-8 UNFACTORED REACTIONS 1ST LCASE MAX/MIN COMPONENT REACTIONS JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL N 1118 620/0 255/0 0/0 0/0 243/0 0/0 H 1118 620/0 255/0 0/0 0/0 243/0 0/0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.84 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED. LOADING TOTAL LOAD CASES: (4) CHORDS WEBS MEMB. MAX. FACTORED FORCE (LBS) FACTORED VERT. LOAD LC1 MAX (PLF) CSI (LC) UNBRAC LENGTH FR-TO MEMB. MAX. FACTORED FORCE (LBS) MAX. FACTORED CSI (LC) FR-TO FROM TO N-A -1296/0 0.0 0.0 0.58 (1) 7.08 A-M 0/1590 0.36 (1) A-B -1173/0 -84.3 -84.3 0.47 (1) 5.23 M-B -856/0 0.37 (1) B-C -1393/0 -84.3 -84.3 0.05 (1) 5.48 B-L -114/0 0.05 (1) C-D -1474/0 -84.3 -84.3 0.44 (1) 4.87 L-C 0/252 0.06 (2) D-E -1474/0 -84.3 -84.3 0.45 (1) 4.84 I-G 0/1557 0.35 (1) E-F -1474/0 -84.3 -84.3 0.45 (1) 4.84 C-K 0/441 0.10 (1) F-G -1108/0 -84.3 -84.3 0.43 (1) 5.43 I-F -911/0 0.53 (1) H-G -1294/0 0.0 0.0 0.93 (1) 7.10 K-D -525/0 0.31 (1) K-F 0/523 0.12 (1) N-M 0/0 -28.0 -28.0 0.15 (2) 10.00 M-L 0/1164 -28.0 -28.0 0.30 (2) 10.00 L-K 0/1163 -28.0 -28.0 0.31 (2) 10.00 K-J 0/1108 -28.0 -28.0 0.36 (2) 10.00 J-I 0/1108 -28.0 -28.0 0.36 (2) 10.00 I-H 0/0 -28.0 -28.0 0.23 (3) 10.00			
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DWG NO. TAM 5141-18
STRUCTURAL
COMPONENT ONLY



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
N - A	2x4	DRY	No.2
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
H - G	2x4	DRY	No.2
N - K	2x4	DRY	No.2
K - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
A	TMV+p	MT20	3.0	4.0
B	TMWW+t	MT20	4.0	6.0
C	TTWW-m	MT20	4.0	4.0 2.00 1.75
D	TTWW-m	MT20	4.0	6.0 1.75 2.50
E	TMWW+w	MT20	2.0	4.0
F	TMWW-l	MT20	4.0	4.0
G	TMWW+p	MT20	4.0	6.0
H	BMV1+p	MT20	3.0	4.0
I	BMWW+t	MT20	4.0	6.0
J	BMWW-t	MT20	4.0	6.0
K	BS-l	MT20	3.0	6.0
L	BMWW-l	MT20	4.0	4.0
M	BMWW+t	MT20	4.0	4.0 2.00 1.50
N	BMVW1+p	MT20	4.0	6.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
		GROSS REACTION		GROSS REACTION		BRG		BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
N	1361	0	1361	0	0	1-8	1-8	1-8	1-8
H	1361	0	1361	0	0	1-8	1-8	1-8	1-8

UNFACTORED REACTIONS

1ST CASE		MAX./MIN. COMPONENT REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
N	1118	620 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0		
H	1118	620 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.32 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-H, B-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED		FACTORED		MAX		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT.	LOAD (PLF)	LC1	MAX	MAX	UNBRAC	MEMB.	FORCE (LBS)	MAX	CS1 (LC)
FR-TO			FROM	TO			LENGTH	FR-TO			
N-A	-112 / 0	0.0	0.0	0.08 (1)	7.81	N-B	-1365 / 0	0.42 (1)			
A-B	0 / 0	-84.3	-84.3	0.17 (1)	10.00	B-M	0 / 935	0.21 (1)			
B-C	-1160 / 0	-84.3	-84.3	0.19 (1)	5.69	M-C	-621 / 0	0.37 (1)			
C-D	-1318 / 0	-84.3	-84.3	0.05 (1)	5.60	C-L	-292 / 0	0.19 (1)			
D-E	-1233 / 0	-84.3	-84.3	0.35 (1)	5.35	L-D	0 / 326	0.07 (2)			
E-F	-1233 / 0	-84.3	-84.3	0.38 (1)	5.32	I-G	0 / 1423	0.32 (1)			
F-G	-883 / 0	-84.3	-84.3	0.34 (1)	6.05	D-J	0 / 214	0.05 (1)			
H-G	-1300 / 0	0.0	0.0	0.30 (1)	5.66	I-F	-950 / 0	0.83 (1)			
						J-E	-479 / 0	0.42 (1)			
						J-F	0 / 576	0.13 (1)			
N-M	0 / 700	-28.0	-28.0	0.37 (2)	10.00						
M-L	0 / 1171	-28.0	-28.0	0.44 (2)	10.00						
L-K	0 / 1101	-28.0	-28.0	0.28 (2)	10.00						
K-J	0 / 1101	-28.0	-28.0	0.28 (2)	10.00						
J-I	0 / 883	-28.0	-28.0	0.29 (2)	10.00						
I-H	0 / 0	-28.0	-28.0	0.19 (3)	10.00						

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. G/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/999 (0.09")
ALLOWABLE DEFL.(TL) = L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/999 (0.15")

CSI: TC=0.36/1.00 (E-F:1), BC=0.44/1.00 (L-M:2), WB=0.83/1.00 (F-I:1), SSI=0.21/1.00 (F-G:1)

DOJ LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

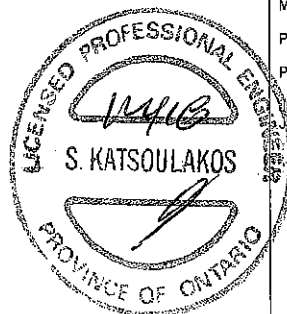
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1867 822 2284 1856

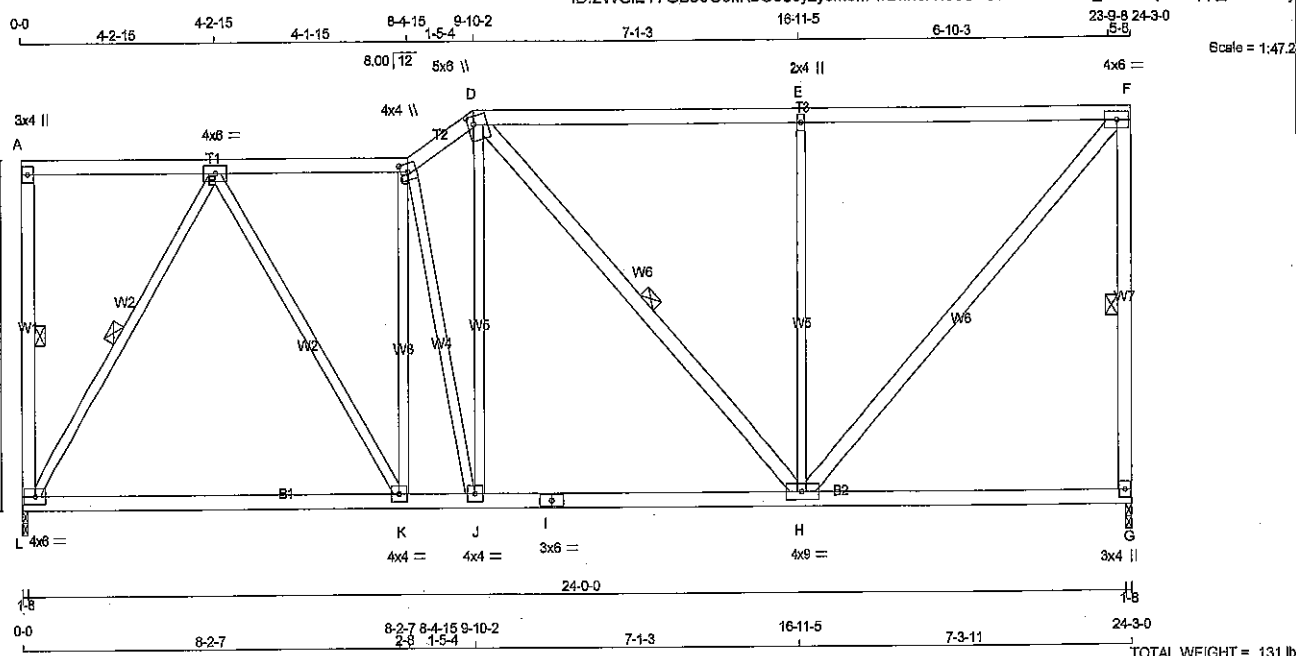
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.89 (M) (INPUT = 0.90)
SI METAL= 0.38 (K) (INPUT = 1.00)



DWG NO. TAM 5142-18
STRUCTURAL
COMPONENT ONLY



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
L - A	2x4	DRY	No.2
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
G - F	2x4	DRY	No.2
L - I	2x4	DRY	No.2
I - G	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
D - H	2x4	DRY	No.2
H - F	2x4	DRY	No.2
DRY; SEASONED LUMBER.			

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
A	TMV+p	MT20	3.0	4.0
B	TMWW-t	MT20	4.0	6.0
C	TTWW+m	MT20	4.0	4.0 2.00 1.75
D	TTWW+m	MT20	5.0	6.0 2.00 1.50
E	TMW+w	MT20	2.0	4.0
F	TMVW-t	MT20	4.0	6.0
G	BMV1+p	MT20	3.0	4.0
H	BMVWW-t	MT20	4.0	9.0
I	BS-t	MT20	3.0	6.0
J	BMWW-t	MT20	4.0	4.0
K	BMWW-t	MT20	4.0	4.0
L	BMVW1-t	MT20	4.0	6.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG
L	1361	0	1361	0	1-8
G	1361	0	1361	0	1-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1118	620 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
G	1118	620 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.74 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF A-L, F-G, D-H, B-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS	
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MAX. CSI (LC)	MEMB.	FORCE (LBS)
FR-TO		FROM	TO	FR-TO	
L-A	-136 / 0	0.0	0.0 0.03 (1)	K-C	-451 / 0
A-B	0 / 0	-84.3	-84.3 0.25 (1)	C-J	-438 / 0
B-C	-1112 / 0	-84.3	-84.3 0.27 (1)	J-D	0 / 453
C-D	-1229 / 0	-84.3	-84.3 0.05 (1)	D-H	-81 / 0
D-E	-974 / 0	-84.3	-84.3 0.83 (1)	H-E	-754 / 0
E-F	-974 / 0	-84.3	-84.3 0.83 (1)	F-H	0 / 1432
G-F	-1261 / 0	0.0	0.0 0.38 (1)	L-B	-1331 / 0
L-K	0 / 701	-28.0	-28.0 0.53 (2)	B-K	0 / 807
K-J	0 / 1119	-28.0	-28.0 0.59 (2)		
J-I	0 / 1029	-28.0	-28.0 0.51 (2)		
I-H	0 / 1029	-28.0	-28.0 0.51 (2)		
H-G	0 / 0	-28.0	-28.0 0.39 (3)		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN.C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL)= L/599 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL)= L/875 (0.30")

CSI: TC=0.83/1.00 (E-F:1), BC=0.59/1.00 (J-K:2), WB=0.95/1.00 (E-H:1), SS=0.30/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1987 822 2284 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (L) (INPUT = 0.90)
JSI METAL= 0.35 (I) (INPUT = 1.00)



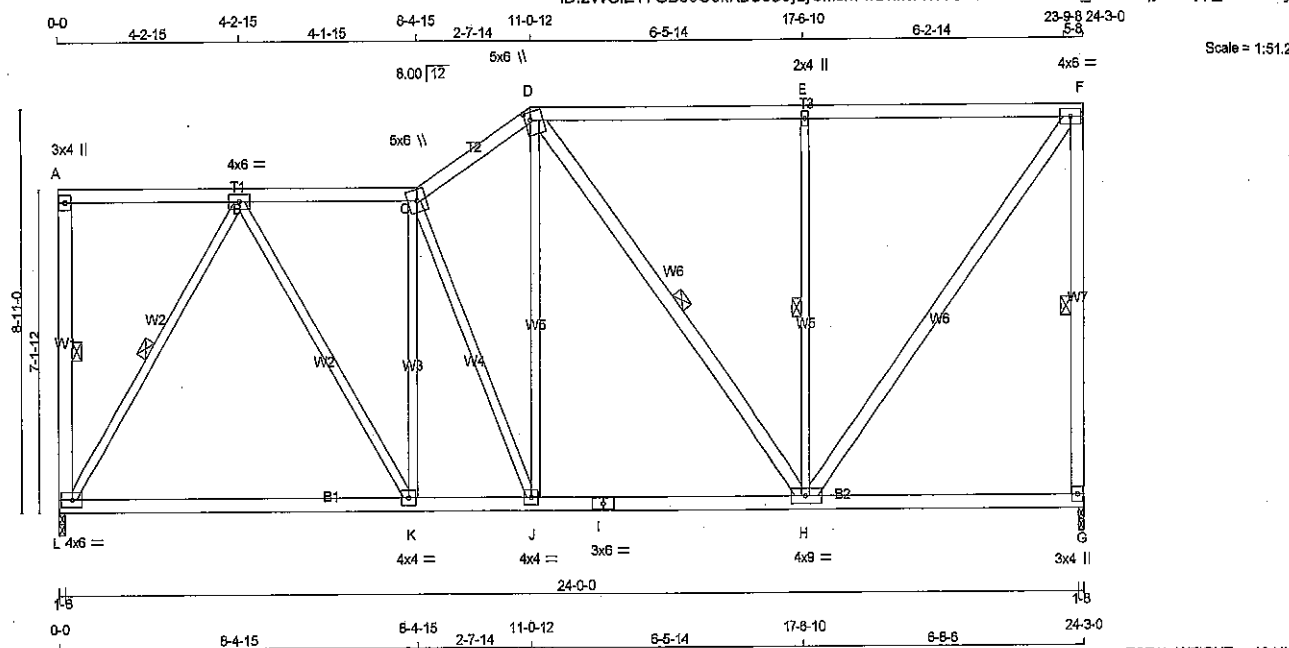
DRWG NO. TAM 5143-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286074	T260	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
L - A	2x4	DRY	No.2	SPF		
A - C	2x4	DRY	No.2	SPF		
C - D	2x4	DRY	No.2	SPF		
D - F	2x4	DRY	No.2	SPF		
G - F	2x4	DRY	No.2	SPF		
L - I	2x4	DRY	No.2	SPF		
I - G	2x4	DRY	No.2	SPF		
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF		
D - H	2x4	DRY	No.2	SPF		
H - F	2x4	DRY	No.2	SPF		

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	5.0		
C	TTWW+m	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMV1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
L	1361	0	1361	0
G	1361	0	1361	0

UNFACTORED REACTIONS

	1ST LCASE	MAX/MIN	COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
L	1118	620/0	255/0	0/0	0/0	243/0	0/0	
G	1118	620/0	255/0	0/0	0/0	243/0	0/0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.51 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF A-L, F-G, B-L, D-H, E-H

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

	CHORDS	WEBS
	MAX. FACTORED MEMB. FORCE (LBS)	MAX. FACTORED MEMB. FORCE (LBS)
FR-TO		
L-A	-136/0	0.0
A-B	0/0	-84.3
B-C	-1110/0	-84.3
C-D	-1141/0	-84.3
D-E	-828/0	-84.3
E-F	-828/0	-84.3
G-F	-1289/0	0.0
L-K	0/700	-28.0
K-J	0/1117	-28.0
J-I	0/955	-28.0
I-H	0/955	-28.0
H-G	0/0	-28.0

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/999 (0.19")
ALLOWABLE DEFL.(TL) = L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/922 (0.32")

CSI: TC=0.67/1.00 (E-F:1), BC=0.58/1.00 (J-K:2), WB=0.57/1.00 (B-L:1), SS=0.27/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (F) (INPUT = 0.90)
JSI METAL= 0.38 (I) (INPUT = 1.00)



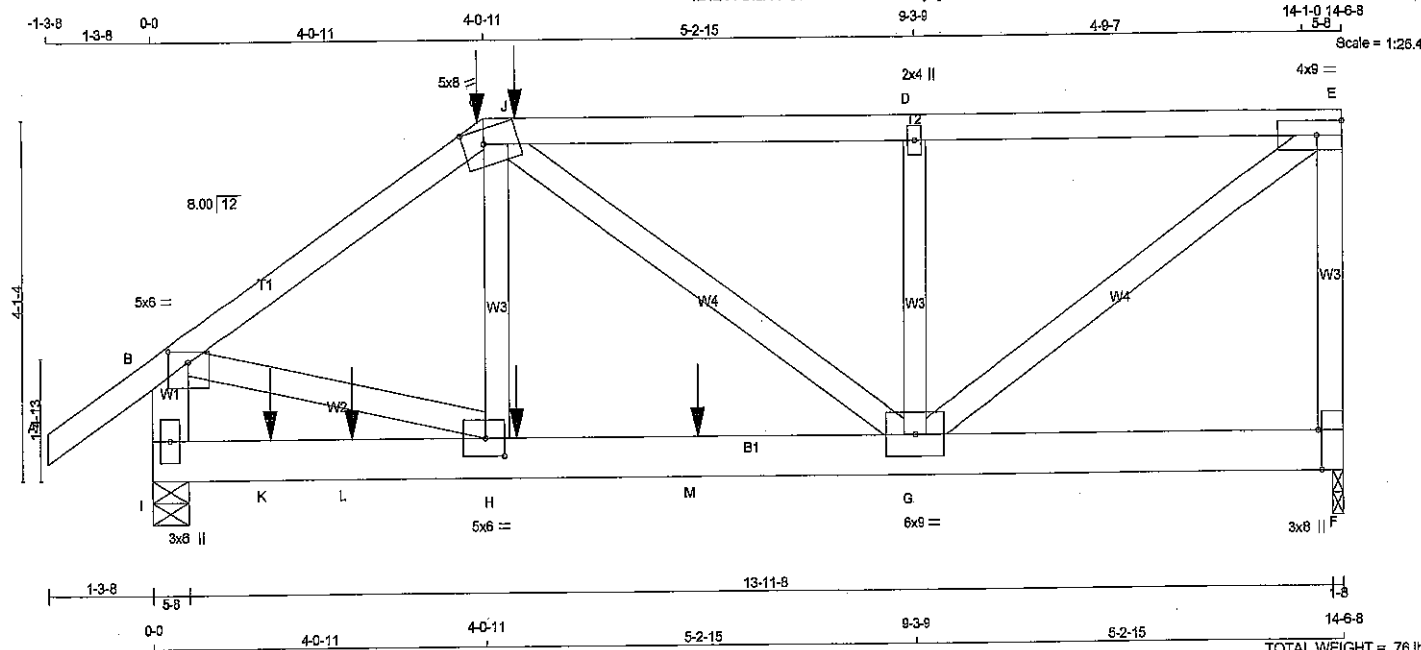
DRWG NO. TAM 5144-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286074	T261	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:2WGIEY7GB8003kKB0cC5zyemsm-wDnh5X4s0SoG7duZ0Rdxwx_xHFXq_WZupp_SDzsYay



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF		
C - E	2x4	DRY	No.2	SPF		
F - E	2x4	DRY	No.2	SPF		
I - B	2x6	DRY	No.2	SPF		
I - F	2x6	DRY	2100F 1.8E	SPF		

ALL WEBS 2x4 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW-p	MT20	5.0	6.0	1.50	3.00
C	TTWW-m	MT20	5.0	8.0	2.00	3.00
D	TMW+w	MT20	2.0	4.0		
E	TMW-l	MT20	4.0	9.0		Edge
F	BMV1+p	MT20	3.0	8.0		Edge 0.50
G	BMWW-l	MT20	6.0	9.0		
H	BMWW-l	MT20	5.0	6.0		2.50 2.75
I	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 244.5 lbs FACTORED DOWN AT 4-0-11, AND 127.4 lbs FACTORED DOWN AT 4-5-4 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 1-5-4, 69.9 lbs FACTORED DOWN AT 2-5-4, AND 69.9 lbs FACTORED DOWN AT 4-5-4, AND 1142.3 lbs FACTORED DOWN AT 6-7-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	BRG	BRG
F	1466	0	1466	0
I	1916	0	1916	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1181	692 / 0	245 / 0	0 / 0	0 / 0	0 / 0	244 / 0	0 / 0
I	1541	908 / 0	318 / 0	0 / 0	0 / 0	0 / 0	318 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.21 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC)	UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)
FR-TO							FR-TO			
A-B	0 / 32	-84.3	-84.3	0.12 (1)	10.00		H-C	0 / 576	0.10 (2)	
B-C	-2189 / 0	-84.3	-84.3	0.36 (1)	4.21		C-G	-32 / 0	0.02 (3)	
C-J	-1826 / 0	-84.3	-84.3	0.52 (1)	4.30		G-D	-553 / 0	0.10 (1)	
J-D	-1826 / 0	-84.3	-84.3	0.52 (1)	4.30		G-E	0 / 2241	0.40 (1)	
D-E	-1826 / 0	-84.3	-84.3	0.52 (1)	4.30		B-H	0 / 1889	0.33 (1)	
F-E	-1466 / 0	0.0	0.0	0.37 (1)	6.72					
I-B	-1937 / 0	0.0	0.0	0.14 (1)	7.19					
I-K	0 / 0	-28.0	-28.0	0.21 (1)	10.00					
K-L	0 / 0	-28.0	-28.0	0.21 (1)	10.00					
L-H	0 / 0	-28.0	-28.0	0.21 (1)	10.00					
H-M	0 / 1839	-28.0	-28.0	0.53 (1)	10.00					
M-G	0 / 1839	-28.0	-28.0	0.53 (1)	10.00					
G-F	0 / 0	-28.0	-28.0	0.15 (1)	10.00					

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	4-0-11	-23	-25		FRONT	VERT.	DEAD
C	4-0-11	-222	-222		FRONT	VERT.	SNOW
H	4-5-4	-40	-70		FRONT	VERT.	TOTAL
J	4-5-4	-127	-127		FRONT	VERT.	TOTAL
K	1-5-4	-40	-70		FRONT	VERT.	TOTAL
L	2-5-4	-40	-70		FRONT	VERT.	TOTAL
M	6-7-8	-1142	-1142		FRONT	VERT.	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 3.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.0/12

*** NON STANDARD GIRDER ***

ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.48")
CALCULATED VERT. DEFL.(LL)= L/999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.48")
CALCULATED VERT. DEFL.(TL)= L/999 (0.14")

CSI: TC=0.52/1.00 (C-D:1), BC=0.53/1.00 (G-H:1),
WB=0.40/1.00 (E-G:1), SS=0.48/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

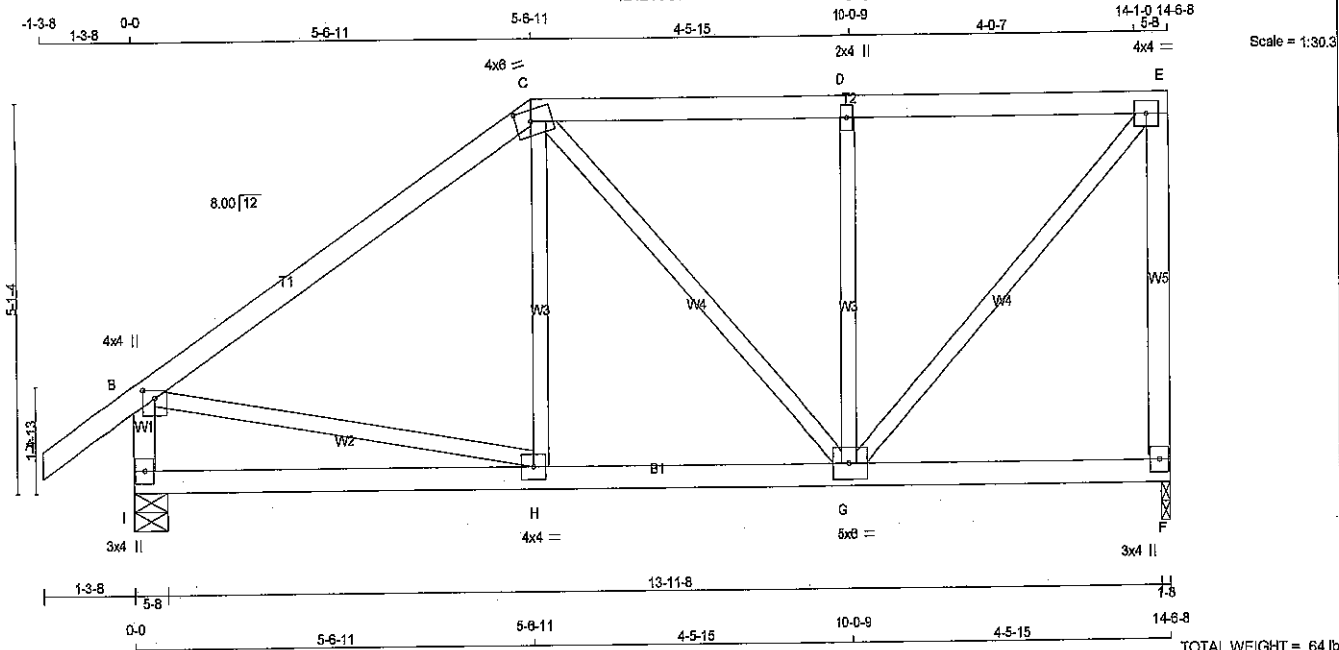
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667
	822	2284	1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (G) (INPUT = 0.90)
JSI METAL= 0.46 (H) (INPUT = 1.00)

DRWG NO. TAM 5145-18
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS SIZE

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
F - E	2x4	DRY	No.2
I - B	2x4	DRY	No.2
I - F	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTWW-m	MT20	4.0	6.0	1.75	2.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	4.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMVW-t	MT20	5.0	6.0		
H	BMVW-t	MT20	4.0	4.0		
I	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	RECORD
F	816	0	816	0	0	1-8	1-8
I	932	0	932	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN.	COMPONENT REACTIONS
F	670	372 / 0	153 / 0
I	749	442 / 0	153 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRAC LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRAC LENGTH
FR-TO	A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	H-C	0 / 221	0.05 (3)
	B-C	-723 / 0	-84.3	-84.3	0.48 (1)	6.25	C-G	-57 / 0	0.04 (3)
	C-D	-579 / 0	-84.3	-84.3	0.28 (1)	6.25	G-D	-466 / 0	0.18 (1)
	D-E	-580 / 0	-84.3	-84.3	0.28 (1)	6.25	G-E	0 / 845	0.19 (1)
	E-F	-762 / 0	0.0	0.0	0.34 (1)	7.81	B-H	0 / 811	0.14 (1)
	I-B	-866 / 0	0.0	0.0	0.09 (1)	7.81			
	I-H	0 / 0	-28.0	-28.0	0.19 (3)	10.00			
	H-G	0 / 601	-28.0	-28.0	0.26 (2)	10.00			
	G-F	0 / 0	-28.0	-28.0	0.12 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 3.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014

- CSA 089-09

- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.48")

CALCULATED VERT. DEFL.(LL) = L/999 (0.04")

ALLOWABLE DEFL.(TL)= L/360 (0.48")

CALCULATED VERT. DEFL.(TL) = L/999 (0.07")

CSI: TC=0.48/1.00 (B-C:1), BC=0.26/1.00 (G-H:2), WB=0.19/1.00 (E-G:1), SSI=0.18/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	818	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (E) (INPUT = 0.90)

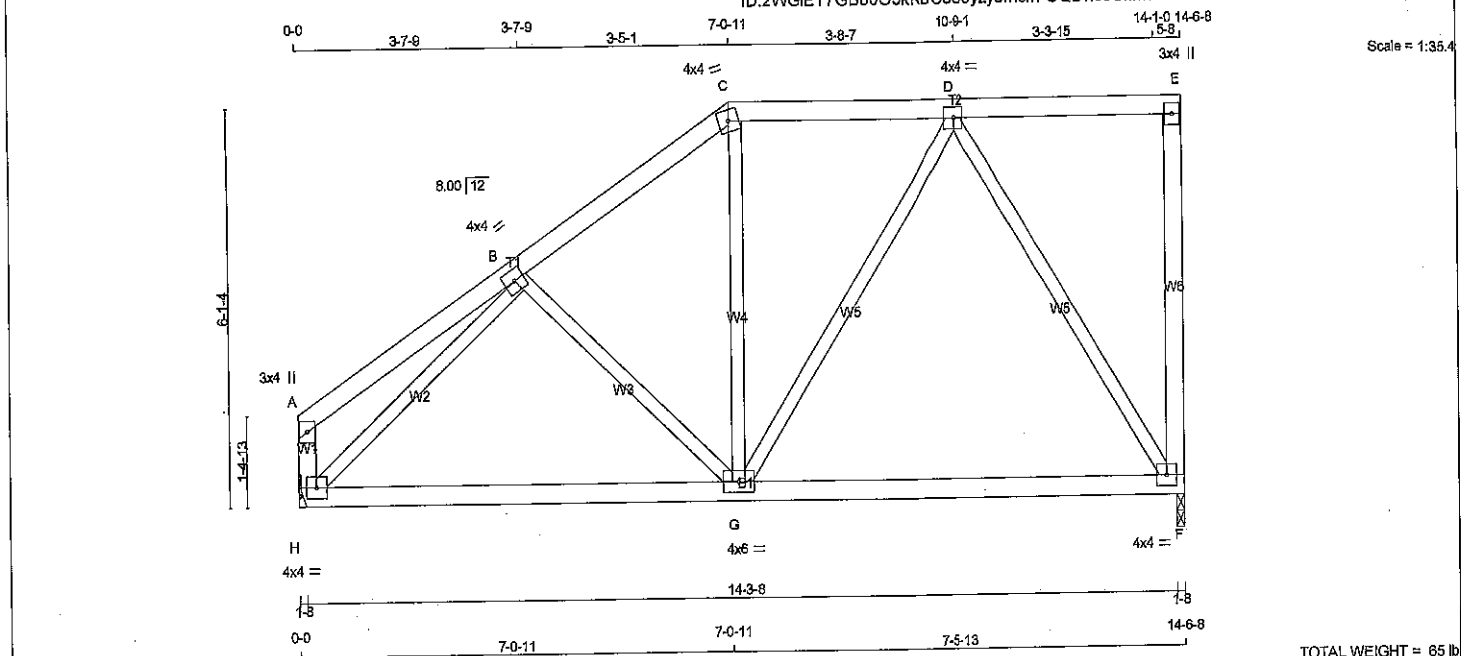
JSI METAL= 0.23 (H) (INPUT = 1.00)



DWG NO. TAM 5146-18

STRUCTURAL

COMPONENT ONLY



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - E	2x4	DRY No.2	SPF
F - E	2x4	DRY No.2	SPF
H - A	2x4	DRY No.2	SPF
H - F	2x4	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-l	MT20	4.0	4.0		
C	TTW-m	MT20	4.0	4.0		
D	TMWW-l	MT20	4.0	4.0		
E	TMV+p	MT20	3.0	4.0		
F	BMVW-l	MT20	4.0	4.0		
G	BMVWW-l	MT20	4.0	6.0		
H	BMVW-l	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
F	816	0	816	0	1-8	1-8
H	816	0	816	0	1-8	1-8

HANGER BY OTHERS
MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS

JT	1ST LCASE MAX./MIN. COMPONENT REACTIONS						
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	670	372 / 0	153 / 0	0 / 0	0 / 0	145 / 0	0 / 0
H	670	372 / 0	153 / 0	0 / 0	0 / 0	145 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

MEMB.	CHORDS			WEBS		
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. MEMB. LENGTH FR-TO	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
A-B	0 / 19	-84.3	-84.3 0.16 (1)	10.00	B-G -145 / 48	0.06 (1)
B-C	-867 / 0	-84.3	-84.3 0.17 (1)	8.25	G-C 0 / 190	0.04 (2)
C-D	-540 / 0	-84.3	-84.3 0.20 (1)	6.25	G-D 0 / 305	0.07 (2)
D-E	0 / 0	-84.3	-84.3 0.19 (1)	10.00	D-F -734 / 0	0.64 (1)
E-F	-122 / 0	0.0	0.0 0.09 (1)	7.81	H-B -898 / 0	0.35 (1)
H-A	-114 / 0	0.0	0.0 0.01 (1)	7.81		
H-G	0 / 643	-28.0	-28.0 0.50 (2)	10.00		
G-F	0 / 400	-28.0	-28.0 0.48 (2)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.48")
CALCULATED VERT. DEFL.(LL) = L/899 (0.09")
ALLOWABLE DEFL.(TL) = L/360 (0.48")
CALCULATED VERT. DEFL.(TL) = L/899 (0.15")

CSI: TC=0.20/1.00 (C-D:1), BC=0.50/1.00 (G-H:2), WB=0.64/1.00 (D-F:1), SS=0.17/1.00 (F-G:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	818	354	1687 822 2264 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (B) (INPUT = 0.90)
JSI METAL= 0.32 (B) (INPUT = 1.00)



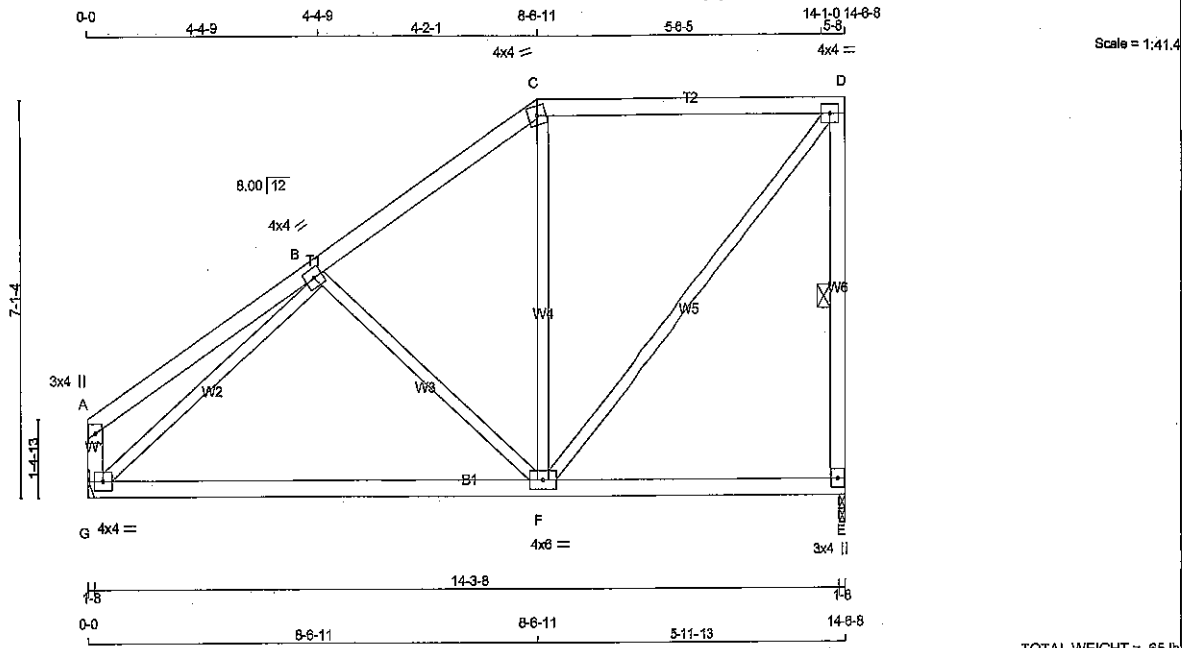
DWG NO. TAM 5147-18
STRUCTURAL
COMPONENT ONLY

JOB NAME 286074	TRUSS NAME T264	QUANTITY 1	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 65 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - A	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMVW-t	MT20	4.0	4.0		
C	TTW-m	MT20	4.0	4.0		
D	TMVW-t	MT20	4.0	4.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMVW-t	MT20	4.0	6.0		
G	BMVW-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
E	816	0	1-8	1-8
G	816	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	670	372 / 0	153 / 0	0 / 0	0 / 0	145 / 0	0 / 0
G	670	372 / 0	153 / 0	0 / 0	0 / 0	145 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-E.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC2)	MAX. FACTORED VERT. LOAD (LC3)	MAX. FACTORED VERT. LOAD (LC4)	MAX. FACTORED VERT. LOAD (LC5)	MAX. FACTORED VERT. LOAD (LC6)	MAX. FACTORED VERT. LOAD (LC7)	MAX. FACTORED VERT. LOAD (LC8)	MAX. FACTORED VERT. LOAD (LC9)	MAX. FACTORED VERT. LOAD (LC10)
FR-TO													
A-B		0 / 24	-84.3	-84.3	0.25 (1)	10.00							
B-C		-562 / 0	-84.3	-84.3	0.20 (1)	6.25							
C-D		-446 / 0	-84.3	-84.3	0.39 (1)	8.25							
E-D		-757 / 0	0.0	0.0	0.17 (1)	6.25							
G-A		-137 / 0	0.0	0.0	0.01 (1)	7.81							
G-F		0 / 632	-28.0	-28.0	0.55 (2)	10.00							
F-E		0 / 0	-28.0	-28.0	0.48 (3)	10.00							

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.48")
CALCULATED VERT. DEFL.(LL) = $L/858$ (0.20")
ALLOWABLE DEFL.(TL) = $L/360$ (0.48")
CALCULATED VERT. DEFL.(TL) = $L/515$ (0.34")

CSI: TC=0.39/1.00 (C-D:1), BC=0.55/1.00 (F-G:2),
WB=0.48/1.00 (B-G:1), SS=0.20/1.00 (F-G:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	618	354	1657
	822	2284	1656

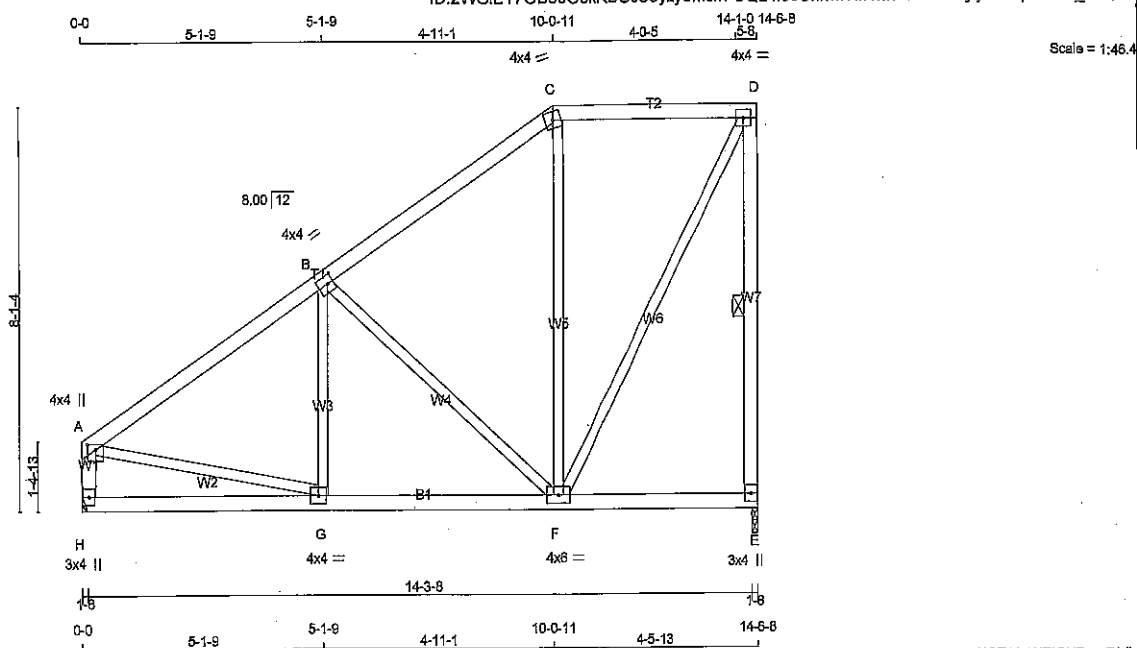
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (F) (INPUT = 0.90)
JSI METAL= 0.29 (B) (INPUT = 1.00)



DRWG NO. TAM 5148-18
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 71 lb

N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	No.2	SPF
C - D	2x4	No.2	SPF
E - D	2x4	No.2	SPF
H - A	2x4	No.2	SPF
H - E	2x4	No.2	SPF

ALL WEBS 2x3 DRY
EXCEPT

DRY, SEASONED LUMBER

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00
B	BMVW-t	MT20	4.0	4.0	2.00	1.50
C	TTW-m	MT20	4.0	4.0		
D	TMVW-t	MT20	4.0	4.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMVWV-t	MT20	4.0	6.0		
G	BMVW-t	MT20	4.0	4.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED		MAXIMUM FACTORED		INPUT	REQ'D		
GROSS REACTION		GROSS REACTION		BRG	BRG		
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	816	0	816	0	0	1-8	1-8
H	816	0	816	0	0	HANGER BY OTHERS	
							MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	670	372 / 0	153 / 0	0 / 0	0 / 0	145 / 0	0 / 0
H	670	372 / 0	153 / 0	0 / 0	0 / 0	145 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY
APPLIED

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-E.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

LOADING
TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MAX. FACTORED		FACTORED			MAX. FACTORED				
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRAC	MEMB.	FORCE (LBS)	MAX	FACTORED	
FR-TO		FROM TO	CSI (LC)	LENGTH	FR-TO				
A-B	-783 / 0	-84.3	-84.3	0.28 (1)	6.25	G-B	0 / 209	0.05 (3)	
B-C	-422 / 0	-84.3	-84.3	0.27 (1)	6.25	B-F	-467 / 0	0.39 (1)	
C-D	-325 / 0	-84.3	-84.3	0.22 (1)	6.25	F-C	-126 / 84	0.16 (1)	
E-D	-767 / 0	0.0	0.0	0.23 (1)	8.25	F-D	0 / 664	0.15 (1)	
H-A	-757 / 0	0.0	0.0	0.08 (1)	7.81	A-G	0 / 680	0.16 (1)	
H-G	0 / 0	-28.0	-28.0	0.17 (3)	10.00				
G-F	0 / 675	-28.0	-28.0	0.25 (2)	10.00				
F-E	0 / 0	-28.0	-28.0	0.14 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	25.6	PSF
		DL =	3.0	PSF
BOT	CH.	LL =	10.5	PSF
		DL =	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.48")
 CALCULATED VERT. DEFL.(LL) = $L/999$ (0.03")
 ALLOWABLE DEFL.(TL) = $L/360$ (0.48")
 CALCULATED VERT. DEFL.(TL) = $L/999$ (0.05")

CSI: TC=0.28/1.00 (A-B:1), BC=0.25/1.00 (F-G:2),
WB=0.39/1.00 (B-F:1), SS=0.17/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

	GRIP(DRY)		SHEAR		SECTION	
	(PSI)	(PSI)	(PLI)	(PLI)	(PLI)	(PLI)
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1867	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (F) (INPUT = 0.90)
JSI METAL= 0.25 (G) (INPUT = 1.00)



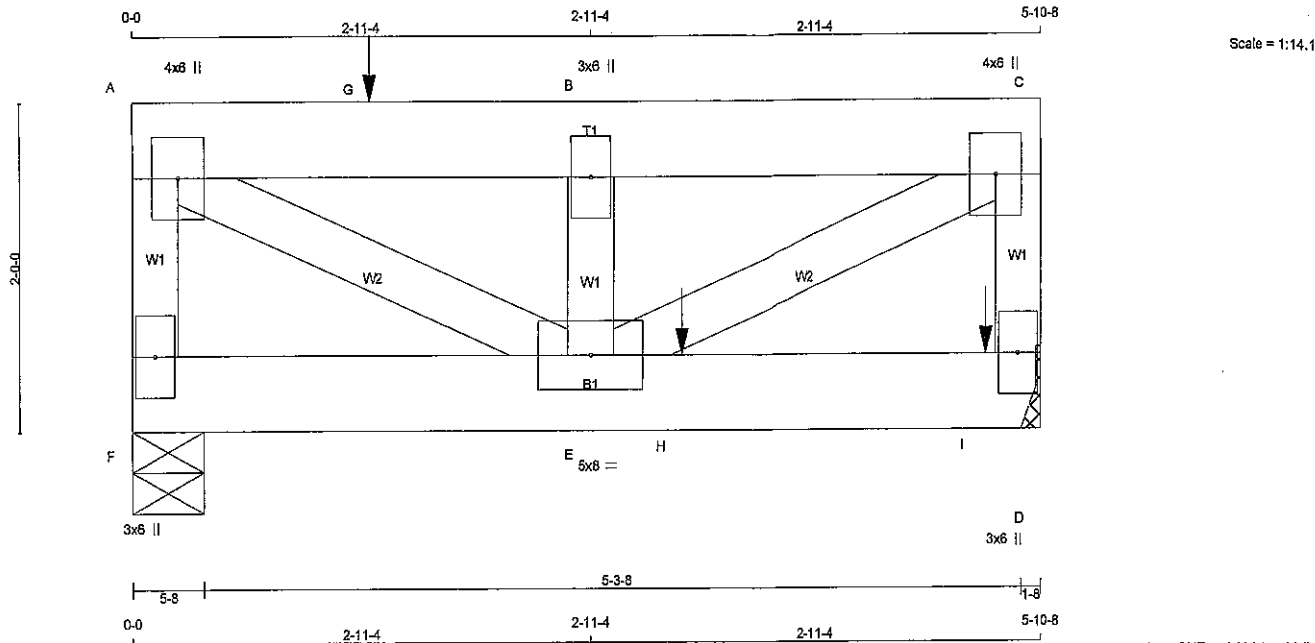
DWG NO. TAM 5149-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
283746	T266	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 31 = 62 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF
A - C	2x6	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
F - A 1	12	TOP
C - D 1	12	TOP
A - C 2	12	SIDE(0.0)
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F - D 2	12	SIDE(122.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x4 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLYS FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0		
B	TMVW+w	MT20	3.0	6.0		
C	TMVW+p	MT20	4.0	6.0		
D	BMV1+p	MT20	3.0	6.0		
E	BMVW+I	MT20	5.0	6.0		
F	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 753.5 lbs FACTORED DOWN AT 1-6-4 ON TOP CHORD, AND 421.0 lbs FACTORED DOWN AT 3-6-4, AND 418.4 lbs FACTORED DOWN AT 5-6-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
F	1082	0	1082	0
D	1170	0	1170	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
F	855	529 / 0	158 / 0	0 / 0	168 / 0
D	936	560 / 0	188 / 0	0 / 0	190 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. CSI (LC)	UNBRAC LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO							FR-TO			
F-A	-1045 / 0	0.0	0.0	0.06 (1)	7.81		A-E	0 / 1318	0.12 (1)	
A-G	-1167 / 0	-84.3	-84.3	0.21 (1)	6.25		E-B	-746 / 0	0.04 (1)	
G-B	-1167 / 0	-84.3	-84.3	0.21 (1)	6.25		E-C	0 / 1318	0.12 (1)	
B-C	-1167 / 0	-84.3	-84.3	0.06 (1)	6.25					
D-C	-882 / 0	0.0	0.0	0.04 (1)	7.81					
F-E	0 / 0	-28.0	-28.0	0.02 (3)	10.00					
E-H	0 / 0	-28.0	-28.0	0.09 (1)	10.00					
H-I	0 / 0	-28.0	-28.0	0.09 (1)	10.00					
I-D	0 / 0	-28.0	-28.0	0.09 (1)	10.00					

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	D/R	TYPE
G	1-6-4	-754	-754		FRONT	VERT	TOTAL
H	3-6-4	-421	-421		FRONT	VERT	TOTAL
I	5-6-4	-418	-418		FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD	=	48.1	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.21 (A-B:1), BC=0.09 (D-E:1), WB=0.12 (A-E:1), SI=0.19 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	618	354	1687
	822	2284	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.58 (A) (INPUT = 0.90)
JSI METAL= 0.18 (C) (INPUT = 1.00)



DRWG NO. TAN 46110-17
STRUCTURAL
COMPONENT ONLY

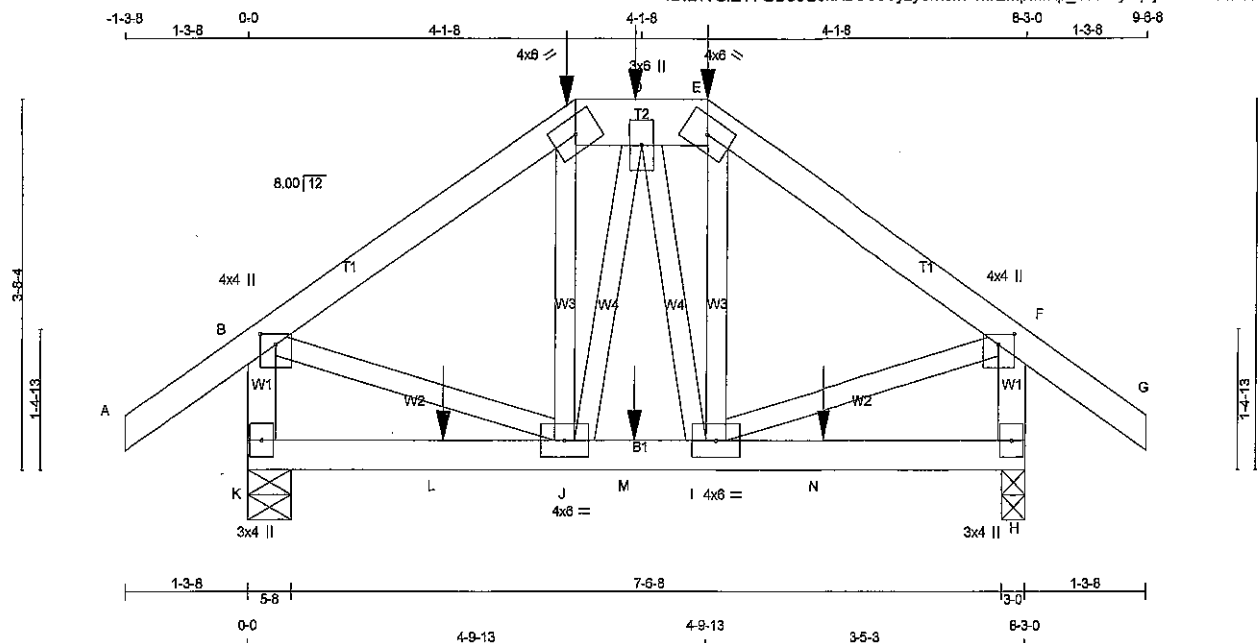
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
283746	T267	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1:23.1



TOTAL WEIGHT = 42 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTW-h	MT20	4.0	6.0		
D	TMVW+t	MT20	3.0	6.0		
E	TTW-h	MT20	4.0	6.0		
F	TMVW+p	MT20	4.0	4.0	1.25	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMVW+t	MT20	4.0	6.0		
J	BMVW+t	MT20	4.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 244.5 lbs FACTORED DOWN AT 3-5-3, AND 244.5 lbs FACTORED DOWN AT 4-9-13, AND 19.7 lbs FACTORED DOWN AT 4-0-12 ON TOP CHORD, AND 5.4 lbs FACTORED DOWN AT 2-0-12, AND 5.4 lbs FACTORED DOWN AT 4-0-12, AND 5.4 lbs FACTORED DOWN AT 6-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX		
K	838	0	838	0	0	5-8	5-8		
H	838	0	838	0	0	3-0	3-0		

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS								
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL		
K	637	435 / 0	90 / 0	0 / 0	0 / 0	112 / 0	0 / 0		
H	637	435 / 0	90 / 0	0 / 0	0 / 0	112 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. (LC)	MEMB.	FORCE (LBS)	MAX. (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 32	-84.3	-84.3 0.12 (1)	10.00	J-C	-53 / 58	0.01 (3)
B-C	-857 / 0	-84.3	-84.3 0.20 (1)	6.25	I-E	-46 / 51	0.01 (3)
C-D	-548 / 0	-84.3	-84.3 0.02 (3)	6.25	B-J	0 / 575	0.14 (1)
D-E	-549 / 0	-84.3	-84.3 0.02 (3)	6.25	I-F	0 / 575	0.14 (1)
E-F	-857 / 0	-84.3	-84.3 0.20 (1)	6.25	J-D	-43 / 24	0.01 (1)
F-G	0 / 32	-84.3	-84.3 0.12 (1)	10.00	D-I	-51 / 29	0.01 (1)
K-B	-794 / 0	0.0	0.0 0.09 (1)	7.81			
H-F	-794 / 0	0.0	0.0 0.09 (1)	7.81			
K-L	0 / 0	-28.0	-28.0 0.08 (2)	10.00			
L-J	0 / 0	-28.0	-28.0 0.08 (2)	10.00			
J-M	0 / 558	-28.0	-28.0 0.13 (1)	10.00			
M-I	0 / 558	-28.0	-28.0 0.13 (1)	10.00			
I-N	0 / 0	-28.0	-28.0 0.08 (2)	10.00			
N-H	0 / 0	-28.0	-28.0 0.08 (2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	3-5-3	-23	-25	-	FRONT	VERT	DEAD
C	3-5-3	-222	-222	-	FRONT	VERT	SNOW
D	4-0-12	-20	-20	-	BACK	VERT	TOTAL
E	4-9-13	-23	-25	-	FRONT	VERT	DEAD
E	4-9-13	-222	-222	-	FRONT	VERT	SNOW
L	2-0-12	-3	-5	-	BACK	VERT	TOTAL
M	4-0-12	-3	-5	-	BACK	VERT	TOTAL
N	6-0-12	-3	-5	-	BACK	VERT	TOTAL



DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. G/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***

ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.28")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.28")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TO=0.20 (E-F:1), BC=0.13 (I-J:1), WB=0.14 (F-I:1), SSI=0.10 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.87 (J) (INPUT = 0.90)
JSI METAL= 0.19 (B) (INPUT = 1.00)

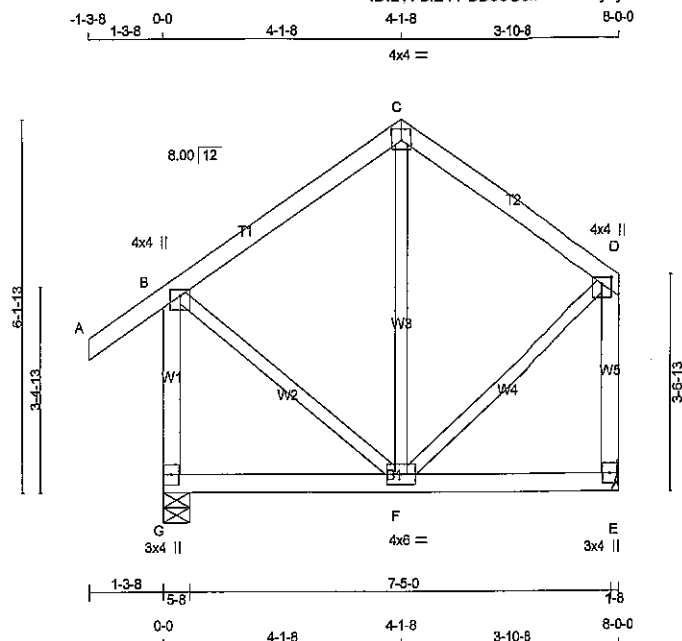
DWG NO. TAM 46111-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
283746	T268A	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1:38.2

TOTAL WEIGHT = 41 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
G - B	2x4	DRY	No.2
E - D	2x4	DRY	No.2
G - E	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	4.0	4.0	1.25 2.00
C	TTW-p	MT20	4.0	4.0	2.25 2.00
D	TMVW+p	MT20	4.0	4.0	1.25 2.00
E	BMV1+p	MT20	3.0	4.0	
F	BMWWW-t	MT20	4.0	6.0	
G	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	VERT	HORZ	DOWN	HORZ	BRG	IN-SX	BRG	IN-SX
G	565	0	0	565	0	5-8	5-8		
E	449	0	0	449	0				

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
G	447	275 / 0	84 / 0	0 / 0	0 / 0	88 / 0	0 / 0	0 / 0
E	369	205 / 0	84 / 0	0 / 0	0 / 0	80 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX. FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX	MAX.	MEMB.	FORCE (LBS)	MAX	MAX.
FR-TO		FROM	TO		LENGTH	FR-TO			
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	F-C	-120 / 95	0.07 (1)	
B-C	-198 / 0	-84.3	-84.3	0.18 (1)	6.25	B-F	0 / 203	0.05 (1)	
C-D	-198 / 0	-84.3	-84.3	0.16 (1)	6.25	F-D	0 / 212	0.05 (1)	
G-B	-520 / 0	0.0	0.0	0.10 (1)	7.81				
E-D	-408 / 0	0.0	0.0	0.09 (1)	7.81				
G-F	0 / 0	-28.0	-28.0	0.13 (3)	10.00				
F-E	0 / 0	-28.0	-28.0	0.13 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.27")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.27")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.18 (B-C:1), BC=0.13 (F-G:3), WB=0.07 (C-F:1), SSI=0.11 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.36 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)



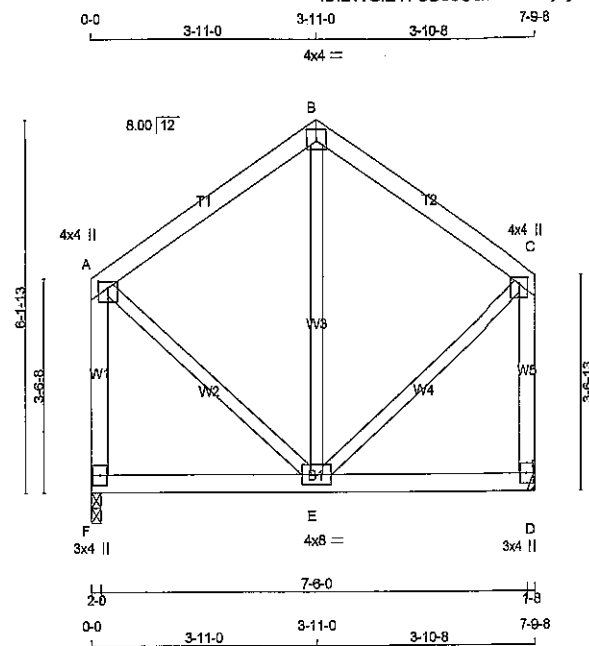
DRW NO. TAM 4612-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
283746	T268B	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 Mitek Industries, Inc. Mon Sep 11 14:28:39 2017 Page 1

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Scale = 1:38.2

TOTAL WEIGHT = 39 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - B	2x4	DRY	No.2
B - C	2x4	DRY	No.2
F - A	2x4	DRY	No.2
D - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW+p	MT20	4.0	4.0	1.25 2.00
B	TTW-p	MT20	4.0	4.0	2.25 2.00
C	TMVW+p	MT20	4.0	4.0	1.25 2.00
D	BMV1+p	MT20	3.0	4.0	
E	BMWW-t	MT20	4.0	6.0	
F	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
F	437	0	437	0	2-0	2-0
D	437	0	437	0	HANGER BY OTHERS	MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
F	359	199 / 0	82 / 0	0 / 0	0 / 0	78 / 0	0 / 0
D	359	199 / 0	82 / 0	0 / 0	0 / 0	78 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	LC1 MAX. (LC)	
FR-TO		FROM TO		FR-TO			
A-B	-186 / 0	-84.3 -84.3	0.17 (1)	E-B	-122 / 89	0.07 (1)	
B-C	-186 / 0	-84.3 -84.3	0.16 (1)	A-E	0 / 200	0.05 (1)	
F-A	-395 / 0	0.0 0.0	0.08 (1)	E-C	0 / 202	0.05 (1)	
D-C	-396 / 0	0.0 0.0	0.08 (1)				
F-E	0 / 0	-28.0 -28.0	0.12 (3)				
E-D	0 / 0	-28.0 -28.0	0.12 (3)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.28")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.28")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.17 (A-B:1), BC=0.12 (D-E:3), WB=0.07 (B-E:1), SS=0.11 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

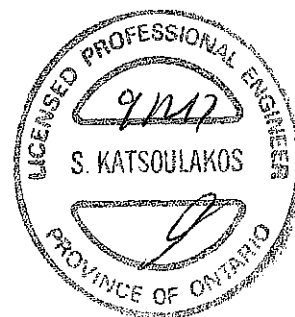
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 822 2284 1656

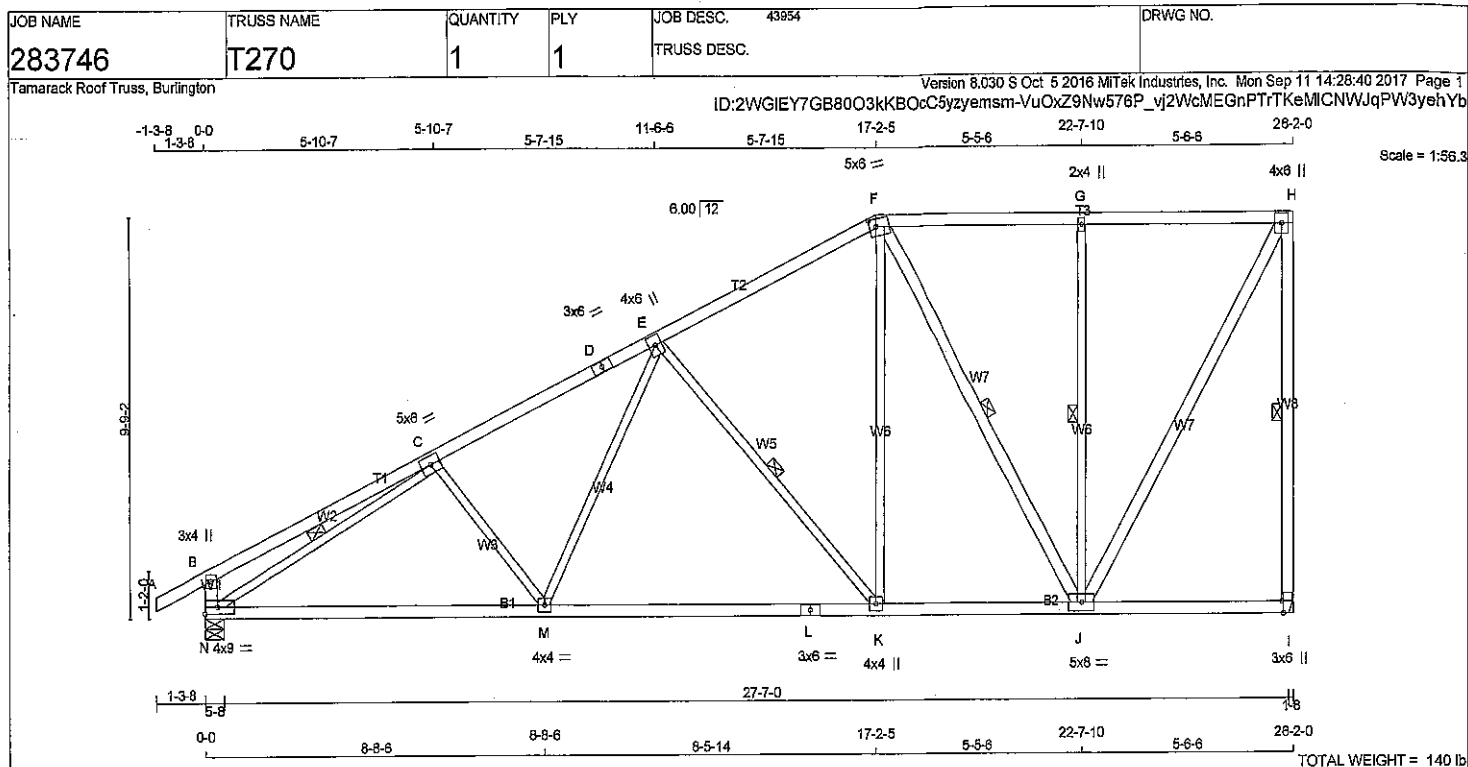
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.28 (C) (INPUT = 0.90)
JSI METAL= 0.07 (A) (INPUT = 1.00)



DWG NO. TAM 96113-17
STRUCTURAL
COMPONENT ONLY



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
I - H	2x4	DRY	No.2
N - B	2x4	DRY	No.2
N - L	2x4	DRY	No.2
L - I	2x4	DRY	No.2

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
J - H	2x4	DRY	No.2	SPF
F - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-I	MT20	5.0	8.0		
D	TS-I	MT20	3.0	6.0		
E	TMWW+I	MT20	4.0	8.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMWW+w	MT20	2.0	4.0		
H	TMWW+p	MT20	4.0	8.0		
I	BMV+I-I	MT20	3.0	6.0	Edge	0.50
J	BMWWW-I	MT20	5.0	8.0		
K	BMWWW-H	MT20	4.0	4.0		
L	BS-I	MT20	3.0	6.0		
M	BMWWW-I	MT20	4.0	4.0		
N	BMWWI-I	MT20	4.0	9.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
I	1581	0	1581	0	0
N	1695	0	1695	0	0

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
I	1298	721 / 0	296 / 0	0 / 0	0 / 0	0 / 0	282 / 0	0 / 0
N	1376	790 / 0	296 / 0	0 / 0	0 / 0	0 / 0	290 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.38 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF H-I, E-K, C-N, F-J, G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	UNBRACED LENGTH
FR-TO		FROM TO		FR-TO			
A-B	0 / 26	-84.3 -84.3	0.11 (1)	C-M	-213 / 87	0.08 (1)	
B-C	0 / 20	-84.3 -84.3	0.35 (1)	M-E	0 / 533	0.12 (2)	
C-D	-2068 / 0	-84.3 -84.3	0.37 (1)	E-K	-816 / 0	0.44 (1)	
D-E	-2068 / 0	-84.3 -84.3	0.37 (1)	K-F	0 / 808	0.18 (1)	
E-F	-1277 / 0	-84.3 -84.3	0.34 (1)	N-C	-2362 / 0	0.75 (1)	
F-G	-782 / 0	-84.3 -84.3	0.33 (1)	J-H	0 / 1550	0.25 (1)	
G-H	-782 / 0	-84.3 -84.3	0.33 (1)	F-J	-702 / 0	0.45 (1)	
H-I	-1516 / 0	0.0 0.0	0.69 (1)	J-G	-573 / 0	0.37 (1)	
N-B	-313 / 0	0.0 0.0	0.03 (1)				
N-M	0 / 1982	-28.0 -28.0	0.76 (2)				
M-L	0 / 1659	-28.0 -28.0	0.73 (2)				
L-K	0 / 1659	-28.0 -28.0	0.73 (2)				
K-J	0 / 1132	-28.0 -28.0	0.40 (2)				
J-I	0 / 0	-28.0 -28.0	0.18 (3)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 45.1 PSF

SPACING = 24.0 IN. CG

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.94")
CALCULATED VERT. DEFL.(LL) = L/999 (0.22")
ALLOWABLE DEFL.(TL) = L/360 (0.94")
CALCULATED VERT. DEFL.(TL) = L/929 (0.36")

CSI: TC=0.69 (H-I), BC=0.76 (M-N2), WB=0.75 (C-N1), SSI=0.23 (G-H1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 822 2284 1656

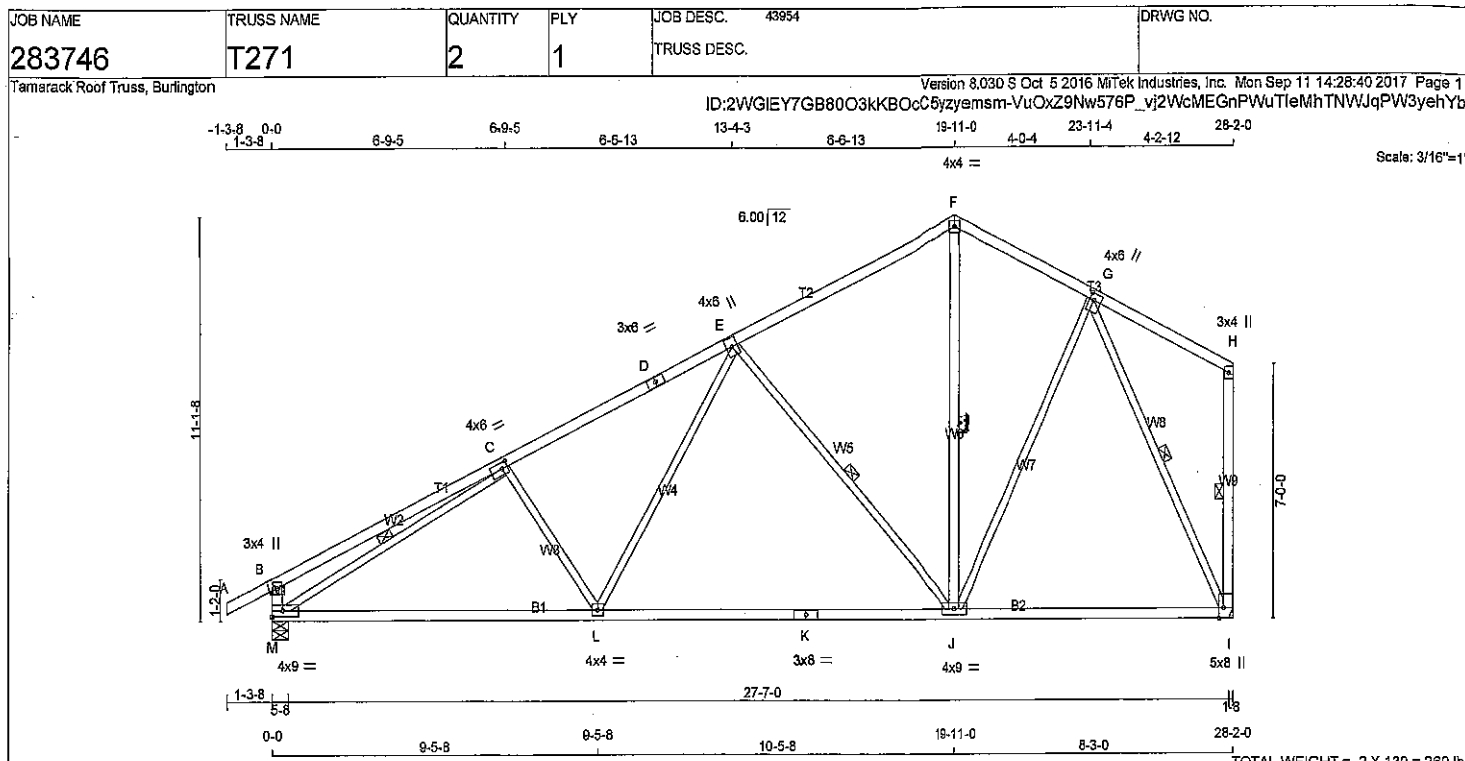
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (N) (INPUT = 0.90)
JSI METAL= 0.58 (C) (INPUT = 1.00)



DWG NO. T270/18-17
STRUCTURAL
COMPONENT ONLY



LUMBER
N. L. G. A. RULES
CHORDS SIZE
A - D 2x4 DRY No.2 SPF
D - F 2x4 DRY No.2 SPF
F - H 2x4 DRY No.2 SPF
M - B 2x4 DRY No.2 SPF
I - H 2x4 DRY No.2 SPF
M - K 2x4 DRY No.2 SPF
K - I 2x4 DRY No.2 SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT J - F 2x4 DRY No.2 SPF

DRY, SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0	2.00	2.00
D	TS-t	MT20	3.0	6.0		
E	TMWW+t	MT20	4.0	6.0		
F	TTW-p	MT20	4.0	4.0		
G	TMWW+t	MT20	4.0	6.0	2.25	1.50
H	TMV+p	MT20	3.0	4.0		
I	BMVW1+t	MT20	5.0	8.0	Edge	1.50
J	BMVWW-t	MT20	4.0	9.0		
K	BS-t	MT20	3.0	8.0		
L	BMVWW-t	MT20	4.0	4.0		
M	BMVW1-t	MT20	4.0	9.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

SUMMATIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT HORZ	DOWN HORZ UPLIFT	IN-SX	IN-SX
M	1695 0	1695 0 0	5-8	5-8
I	1581 0	1581 0 0	HANGER BY OTHERS	
MIN. SEAT SIZE: 1-8				

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1376	790 / 0	296 / 0	0 / 0	0 / 0	290 / 0	0 / 0
I	1298	721 / 0	296 / 0	0 / 0	0 / 0	282 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.28 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-J, C-M, H-I, G-I. F-5

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 26	-84.3 -84.3	0.11 (1)	C-L	-323 / 63	0.14 (1)	
B-C	0 / 24	-84.3 -84.3	0.49 (1)	L-E	0 / 864	0.15 (2)	
C-D	-2021 / 0	-84.3 -84.3	0.50 (1)	E-J	-966 / 0	0.70 (1)	
D-E	-2021 / 0	-84.3 -84.3	0.50 (1)	J-F	0 / 521	0.08 (2)	
E-F	-1003 / 0	-84.3 -84.3	0.45 (1)	F-G	0 / 549	0.12 (1)	
F-G	-988 / 0	-84.3 -84.3	0.19 (1)	G-I	-2347 / 0	0.97 (1)	
G-H	0 / 18	-84.3 -84.3	0.23 (1)	I-H	-1504 / 0	0.99 (1)	
H-I	-343 / 0	0.0 0.0	0.03 (1)				
I-H	-133 / 0	0.0 0.0	0.03 (1)				
M-L	0 / 1987	-28.0 -28.0	0.89 (2)				
L-K	0 / 1510	-28.0 -28.0	0.84 (2)				
K-J	0 / 1510	-28.0 -28.0	0.84 (2)				
J-I	0 / 640	-28.0 -28.0	0.84 (2)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.94")
CALCULATED VERT. DEFL.(LL) = L/999 (0.28")
ALLOWABLE DEFL.(TL)= L/360 (0.94")
CALCULATED VERT. DEFL.(TL) = L/726 (0.47")

CSI: TC=0.50 (C-E:1), BC=0.89 (L-M:2), WB=0.99 (G-I:1), SSI=0.23 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

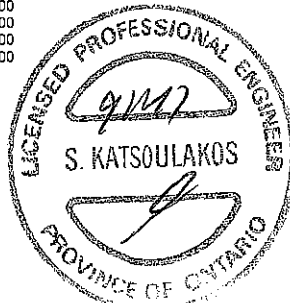
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.90 (C) (INPUT = 0.90)
JSI METAL = 0.74 (K) (INPUT = 1.00)

DWG NO. TAM46116-17

STRUCTURAL

COMPONENT ONLY



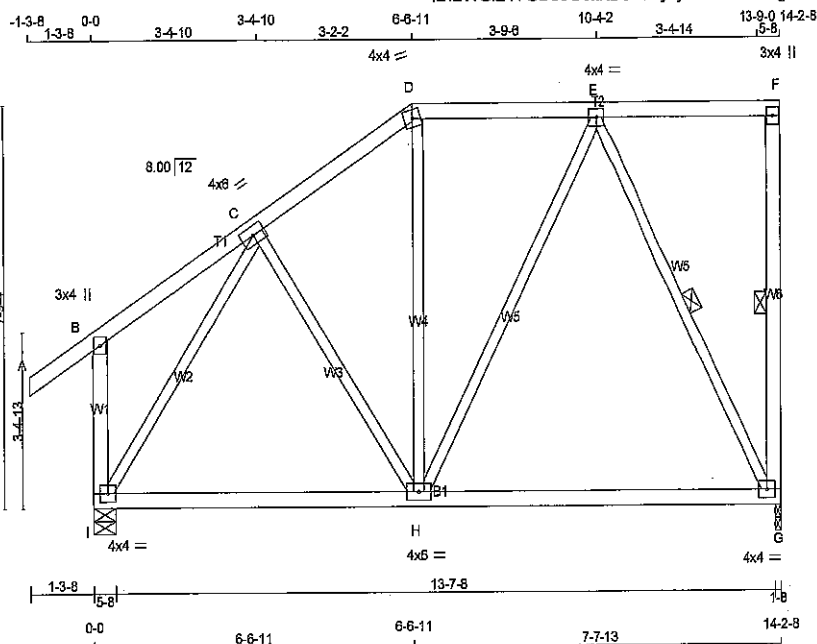
DWG NO. TAM46116-17
 STRUCTURAL
 COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286075	T273A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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Scale = 1:44.6

TOTAL WEIGHT = 76 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW-t	MT20	4.0	4.0		
H	BMWW-t	MT20	4.0	6.0		
I	BMVW-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
G	VERT 797	DOWN 797	1-8	1-8
I	HORIZ 913	HORIZ 913	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN	COMPONENT REACTIONS	WIND	DEAD	SOIL
G	655	364 / 0	149 / 0	0 / 0	142 / 0	0 / 0
I	733	434 / 0	149 / 0	0 / 0	150 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, E-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)
FR-TO									
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	C-H	0 / 109	0.02 (3)	
B-C	0 / 18	-84.3	-84.3	0.15 (1)	10.00	H-D	0 / 132	0.03 (3)	
C-D	-498 / 0	-84.3	-84.3	0.15 (1)	6.25	H-E	0 / 261	0.08 (2)	
D-E	-399 / 0	-84.3	-84.3	0.21 (1)	6.25	E-G	-665 / 0	0.32 (1)	
E-F	0 / 0	-84.3	-84.3	0.20 (1)	10.00	I-C	-738 / 0	0.48 (1)	
G-F	-124 / 0	0.0	0.0	0.03 (1)	6.25				
I-B	-221 / 0	0.0	0.0	0.04 (1)	7.81				
I-H	0 / 388	-28.0	-28.0	0.46 (2)	10.00				
H-G	0 / 305	-28.0	-28.0	0.45 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.8 PSF
DL	= 3.0 PSF
BOT CH. LL	= 10.5 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.47")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.10")
ALLOWABLE DEFL.(TL) = $L/360$ (0.47")
CALCULATED VERT. DEFL.(TL) = $L/983$ (0.17")

CSI: TC=0.21/1.00 (D-E:1), BC=0.46/1.00 (H-I:2), WB=0.48/1.00 (G-I:1), SS=0.18/1.00 (G-H:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667
	822	2284	1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (C) (INPUT = 0.90)
JSI METAL= 0.17 (C) (INPUT = 1.00)



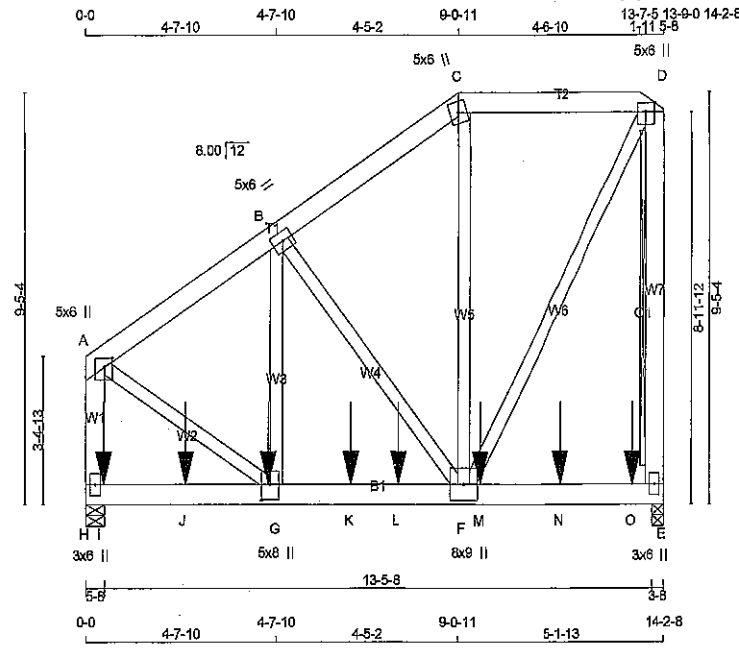
DRWG NO. TAM 5132-18
STRUCTURAL
COMPONENT ONLY

JOB NAME 286075	TRUSS NAME T274A	QUANTITY 1	PLY 3	JOB DESC. TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 3 X 115 = 345 lb

LUMBER					
N. L. G. A. RULES					
CHORDS		SIZE	LUMBER		DESCR
A - C	2x8	DRY	No.2	SPF	
C - D	2x8	DRY	No.2	SPF	
H - A	2x8	DRY	No.2	SPF	
E - D	2x8	DRY	No.2	SPF	
H - E	2x8	DRY	No.2	SPF	
ALL WEBS EXCEPT		2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
A-C	2	12
C-D	2	12
H-A	2	7
E-D	2	12
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
H-E	2	7
WEBS: (0.122"x3") SPIRAL NAILS		
2x4	1	6

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PILES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	2.50 2.25
B	TMVW-t	MT20	5.0	6.0	
C	TTW+m	MT20	5.0	6.0	
D	TMVW+p	MT20	5.0	8.0	Edge 2.75
E	BMV1+p	MT20	3.0	6.0	
F	BMVW+H	MT20	8.0	9.0	
G	BMVW+H	MT20	5.0	8.0	4.25 2.50
H	BMV1+p	MT20	3.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION	FACTORED GROSS REACTION	INPUT BRG	REQD BRG
VERT	HORZ	UPLIFT	IN-SX
JT 7431	0	7431	0
H 7431	0	7431	0
E 7055	0	7055	0

UNFACTORED REACTIONS

1ST LCASE	MAX MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW
H 6007	3488 / 0	1266 / 0
E 5731	3282 / 0	1238 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.14 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x8 DRY SPF No.2 T-BRACE AT D-E

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. FACTORED CSI (LC)	WEBS MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO		FROM	TO	FR-TO	
A-B	-4842 / 0	-84.3	-84.3	0.05 (1)	6.14
B-C	-3359 / 0	-84.3	-84.3	0.04 (1)	6.25
C-D	-2802 / 0	-84.3	-84.3	0.07 (1)	6.25
H-A	-5392 / 0	0.0	0.0	0.20 (1)	7.43
E-D	-5232 / 0	0.0	0.0	0.42 (1)	7.81
H-I	0 / 0	-28.0	-28.0	0.34 (1)	10.00
I-J	0 / 0	-28.0	-28.0	0.34 (1)	10.00
J-G	0 / 0	-28.0	-28.0	0.34 (1)	10.00
G-K	0 / 4039	-28.0	-28.0	0.64 (1)	10.00
K-L	0 / 4039	-28.0	-28.0	0.48 (1)	10.00
L-F	0 / 4039	-28.0	-28.0	0.64 (1)	10.00
F-M	0 / 0	-28.0	-28.0	0.48 (1)	10.00
M-N	0 / 0	-28.0	-28.0	0.48 (1)	10.00
N-O	0 / 0	-28.0	-28.0	0.48 (1)	10.00
O-E	0 / 0	-28.0	-28.0	0.48 (1)	10.00

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
G	4-5-4	-1667	-1667	---	FRONT	VERT	TOTAL
I	5-4	-1675	-1675	---	FRONT	VERT	TOTAL
J	2-5-4	-1667	-1667	---	FRONT	VERT	TOTAL
K	6-5-4	-1667	-1667	---	FRONT	VERT	TOTAL
L	7-7-4	-1553	-1553	---	FRONT	VERT	TOTAL
M	9-7-4	-1553	-1553	---	FRONT	VERT	TOTAL
N	11-7-4	-1553	-1553	---	FRONT	VERT	TOTAL
O	13-5-4	-1556	-1556	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 25.6	PSF
	DL	= 3.0	PSF
BOT CH.	LL	= 10.5	PSF
	DL	= 7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.47")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.47")
CALCULATED VERT. DEFL.(TL) = L/999 (0.09")

CSI: TC=0.42/1.00 (D-E:1), BC=0.64/1.00 (F-G:1), WB=0.52/1.00 (B-F:1), SS=0.60/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	618	354	1887
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (D) (INPUT = 0.90)
JSI METAL= 0.35 (G) (INPUT = 1.00)

DWG NO. TAM 5133-18
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286075	T274A	1	3	TRUSS DESC.	

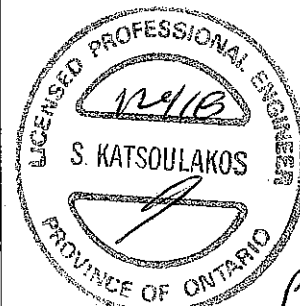
Tamarack Roof Truss, Burlington

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HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1875.1 lbs FACTORED DOWN AT 5-4, 1667.2 lbs FACTORED DOWN AT 2-5-4, 1667.2 lbs FACTORED DOWN AT 4-5-4, 1667.2 lbs FACTORED DOWN AT 6-5-4, 1552.9 lbs FACTORED DOWN AT 7-7-4, 1552.9 lbs FACTORED DOWN AT 9-7-4, AND 1552.9 lbs FACTORED DOWN AT 11-7-4, AND 1556.1 lbs FACTORED DOWN AT 13-5-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.



DWG NO. TAM 5133-18
 STRUCTURAL
 COMPONENT ONLY

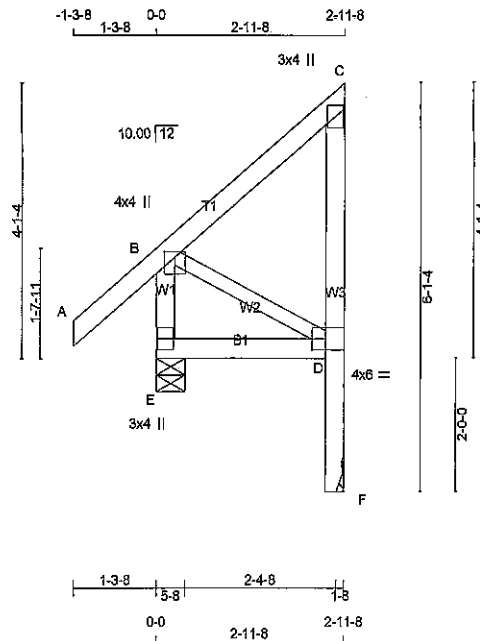
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
283746	T276	4	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 4 X 20 = 79 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4 DRY	No.2	SPF
F - C	2x4 DRY	No.2	SPF
E - B	2x4 DRY	No.2	SPF
E - D	2x4 DRY	No.2	SPF
ALL WEBS	2x3 DRY	No.2	SPF
DRY: SEASONED LUMBER.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	HORZ	BRG	IN-SX	BRG	IN-SX
F	166	0	0	166	0	0	5-8	5-8	5-8
E	283	0	0	283	0	0	5-8	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	136	76 / 0	31 / 0	0 / 0	0 / 0	30 / 0	0 / 0	0 / 0
E	216	147 / 0	31 / 0	0 / 0	0 / 0	38 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	CS1 (LC)	MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	CS1 (LC)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 37	-84.3	-84.3	0.12 (1)	10.00	B-D	0 / 0
B-C	0 / 0	-84.3	-84.3	0.13 (1)	10.00		
F-D	-166 / 0	0.0	0.0	0.02 (1)	7.81		
D-C	-125 / 0	0.0	0.0	0.04 (1)	7.81		
E-B	-241 / 0	0.0	0.0	0.03 (1)	7.81		
E-D	0 / 0	-28.0	-28.0	0.07 (3)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.13 (B-C:1), BC=0.07 (D-E:3), WB=0.00 (B-D:1), SSI=0.07 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1657 822 2284 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)



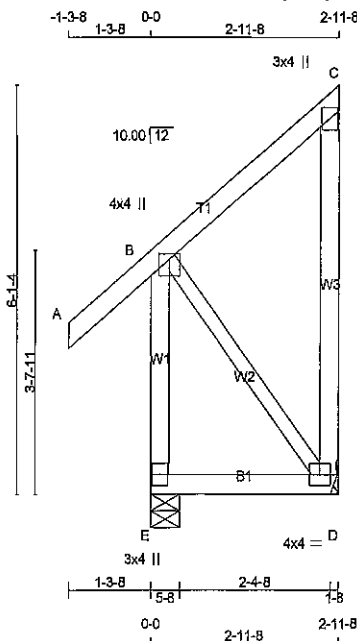
DWG NO. TAM 46121-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43964	DRWG NO.
283746	T277	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1:34.6

TOTAL WEIGHT = 23 lb

LUMBER				DESCR.	
N.L.G.A. RULES					
CHORDS	SIZE		LUMBER		
A - C	2x4	DRY	No.2	SPF	
D - C	2x4	DRY	No.2	SPF	
E - B	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	
ALL WEBS 2x3 DRY				No.2	SPF
DRY: SEASONED LUMBER.					

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	4.0	4.0	1.00 2.00
C	TMV+p	MT20	3.0	4.0	
D	BMVW1-t	MT20	4.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
D	166	0	166	0	0	HANGER BY OTHERS
E	283	0	283	0	0	MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
D	136	76 / 0	31 / 0	0 / 0	0 / 0	30 / 0	0 / 0
E	216	147 / 0	31 / 0	0 / 0	0 / 0	38 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS		WEBS		MEMB.	MAX. FACTORED	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)		MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (PLF)
FR-TO					FR-TO		
A-B	0 / 37	-84.3	-84.3	0.12 (1)	B-D	0 / 0	0.00 (1)
B-C	0 / 0	-84.3	-84.3	0.13 (1)			
D-C	-125 / 0	0.0	0.0	0.09 (1)			
E-B	-241 / 0	0.0	0.0	0.05 (1)			
E-D	0 / 0	-28.0	-28.0	0.07 (3)			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.13 (B-C:1), BC=0.07 (D-E:3), WB=0.00 (B-D:1), SSI=0.07 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

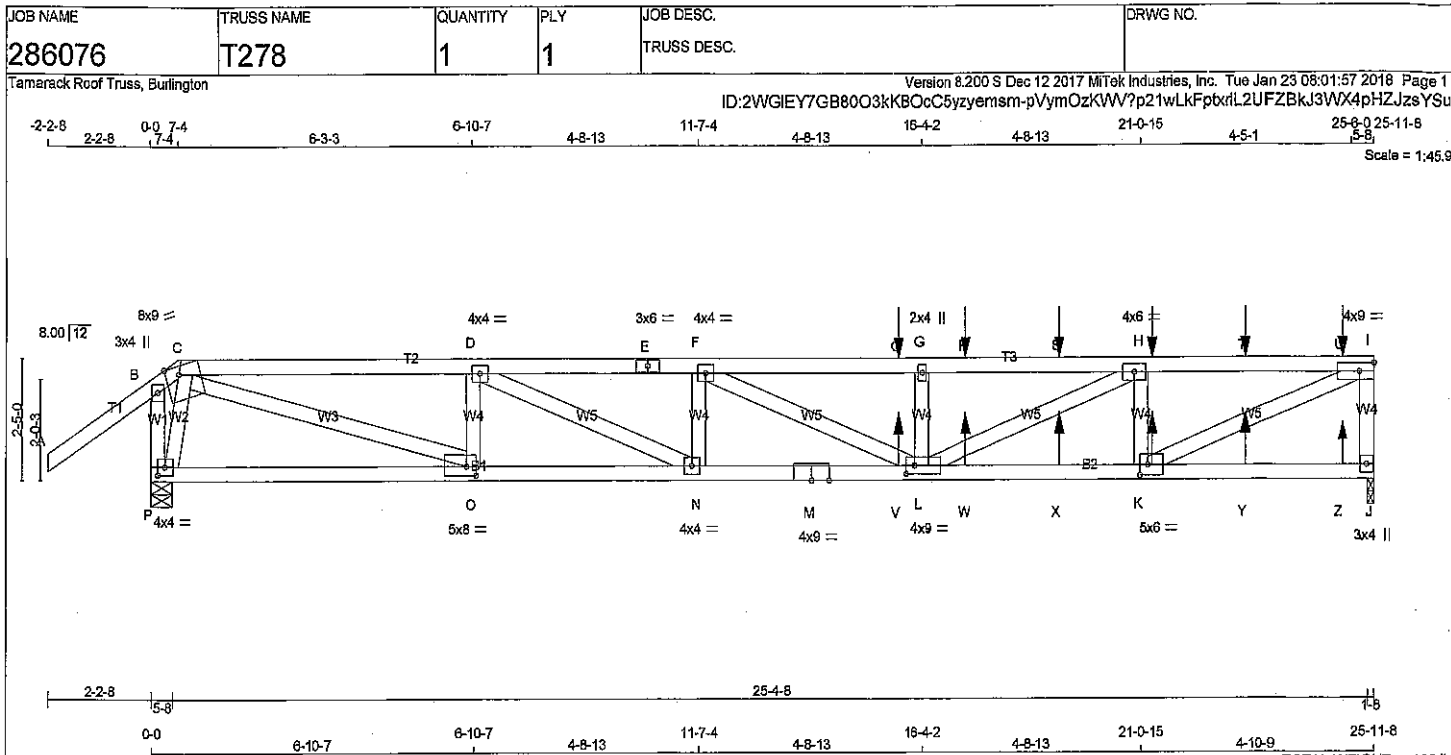
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.19 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)



DWONG, TAN 4612217
STRUCTURAL
COMPONENT ONLY



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - E	2x4	DRY No.2	SPF
E - I	2x4	DRY No.2	SPF
J - L	2x4	DRY No.2	SPF
P - B	2x4	DRY No.2	SPF
P - M	2x4	DRY No.2	SPF
M - J	2x4	DRY No.2	SPF

ALL WEBS 2x4 DRY No.2 SPF
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TTWW-m	MT20	8.0	9.0	Edge	3.50
D	TMWW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMWW-w	MT20	2.0	4.0		
H	TMWW-t	MT20	4.0	6.0		
I	TMWW-t	MT20	4.0	9.0	Edge	
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0	2.50	2.00
L	BMWW-w	MT20	4.0	9.0	2.00	2.50
M	BS-t	MT20	4.0	9.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMWW-t	MT20	5.0	8.0	2.25	2.50
P	BMWW-t	MT20	4.0	4.0	2.00	1.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 2.4 lbs FACTORED DOWN AND 111.2 lbs FACTORED UP AT 15-9-12, 2.4 lbs FACTORED DOWN AND 111.2 lbs FACTORED UP AT 17-3-12, 2.4 lbs FACTORED DOWN AND 111.2 lbs FACTORED UP AT 19-3-12, 2.4 lbs FACTORED DOWN AND 111.2 lbs FACTORED UP AT 21-3-12, AND 2.4 lbs FACTORED DOWN AND 111.2 lbs FACTORED UP AT 23-3-12, AND 2.4 lbs FACTORED DOWN AND 97.6 lbs FACTORED UP AT 25-3-12 ON TOP CHORD, AND 1.0 lbs FACTORED DOWN AND 9.5 lbs FACTORED UP AT 15-9-12, 1.0 lbs FACTORED DOWN AND 9.5 lbs FACTORED UP AT 17-3-12, 1.0 lbs FACTORED DOWN AND 9.5 lbs FACTORED UP AT 19-3-12, AND 1.0 lbs FACTORED DOWN AND 9.5 lbs FACTORED UP AT 21-3-12, AND 1.0 lbs FACTORED DOWN AND 9.5 lbs FACTORED UP AT 23-3-12, AND 8.1 lbs FACTORED DOWN AND 5.4 lbs FACTORED UP AT 25-3-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	REQD
J	1411	0	1411	0	0	1-8	1-8
P	1647	0	1647	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX	MIN	COMPONENT REACTIONS
J	COMBINED	629 / 0	284 / 0	0 / 0 0 / 0 281 / 0 0 / 0
P	COMBINED	778 / 0	275 / 0	0 / 0 0 / 0 274 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, P

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.81 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

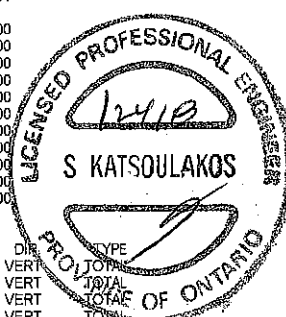
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (7)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 53	-84.3 -84.3	0.34 (1)	10.00	C-D	-901 / 0	0.11 (1)
B-C	-197 / 0	-84.3 -84.3	0.33 (1)	6.25	P-C	-1035 / 0	0.13 (1)
C-D	-3513 / 0	-84.3 -84.3	0.86 (1)	2.98	C-O	0 / 3400	0.60 (1)
D-E	-4325 / 0	-84.3 -84.3	0.82 (1)	2.81	K-I	0 / 2978	0.53 (1)
E-F	-4325 / 0	-84.3 -84.3	0.82 (1)	2.81	D-N	0 / 890	0.16 (1)
F-G	-4091 / 0	-84.3 -84.3	0.54 (1)	3.07	K-H	-1067 / 0	0.13 (1)
G-H	-4091 / 0	-84.3 -84.3	0.62 (1)	2.96	N-F	-251 / 15	0.03 (1)
H-I	-4091 / 0	-84.3 -84.3	0.62 (1)	2.96	L-H	0 / 1493	0.28 (1)
I-J	-2729 / 0	-84.3 -84.3	0.44 (1)	3.77	F-L	-593 / 0	0.19 (5)
J-U	-2729 / 0	-84.3 -84.3	0.44 (1)	3.77	L-G	-395 / 0	0.05 (1)
U-I	-2729 / 0	-84.3 -84.3	0.44 (1)	3.77			
J-I	-1358 / 0	0.0 0.0	0.18 (1)	6.93			
P-B	-571 / 0	0.0 0.0	0.07 (1)	7.81			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE
H	21-3-12	-1	-2	111	BACK	VERT	TOTAL
K	21-3-12	10	-0	10	BACK	VERT	TOTAL
Q	15-9-12	-1	-2	111	BACK	VERT	TOTAL
R	17-3-12	-1	-2	111	BACK	VERT	TOTAL
S	19-3-12	-1	-2	111	BACK	VERT	TOTAL
T	23-3-12	-1	-2	111	BACK	VERT	TOTAL
U	25-3-12	-1	-2	98	BACK	VERT	TOTAL
V	15-9-12	10	-0	10	BACK	VERT	TOTAL
W	17-3-12	10	-0	10	BACK	VERT	TOTAL
X	19-3-12	10	-0	10	BACK	VERT	TOTAL



DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.87")
CALCULATED VERT. DEFL.(LL) = L/895 (0.35")
ALLOWABLE DEFL.(TL) = L/360 (0.87")
CALCULATED VERT. DEFL.(TL) = L/554 (0.56")

CSI: TC=0.88/1.00 (C-D:1), BC=0.87/1.00 (L-N:1), WB=0.60/1.00 (C-O:1), SI=0.27/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MT20	615	354	1667 622 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (O) (INPUT = 0.90)
JSI METAL= 0.89 (M) (INPUT = 1.00)

DWG NO. TAM 5110
STRUCTURAL
COMPONENT ONLY
CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286076	T278	1	1	TRUSS DESC.	

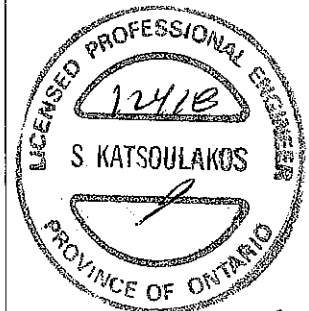
Tamarack Roof Truss, Burlington

Version 8.200 S Dec 12 2017 Mitek Industries, Inc. Tue Jan 23 08:01:57 2018 Page 2

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FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
Y	23-3-12	10	-0	10	BACK	VERT	TOTAL
Z	25-3-12	5	-8	5	BACK	VERT	TOTAL



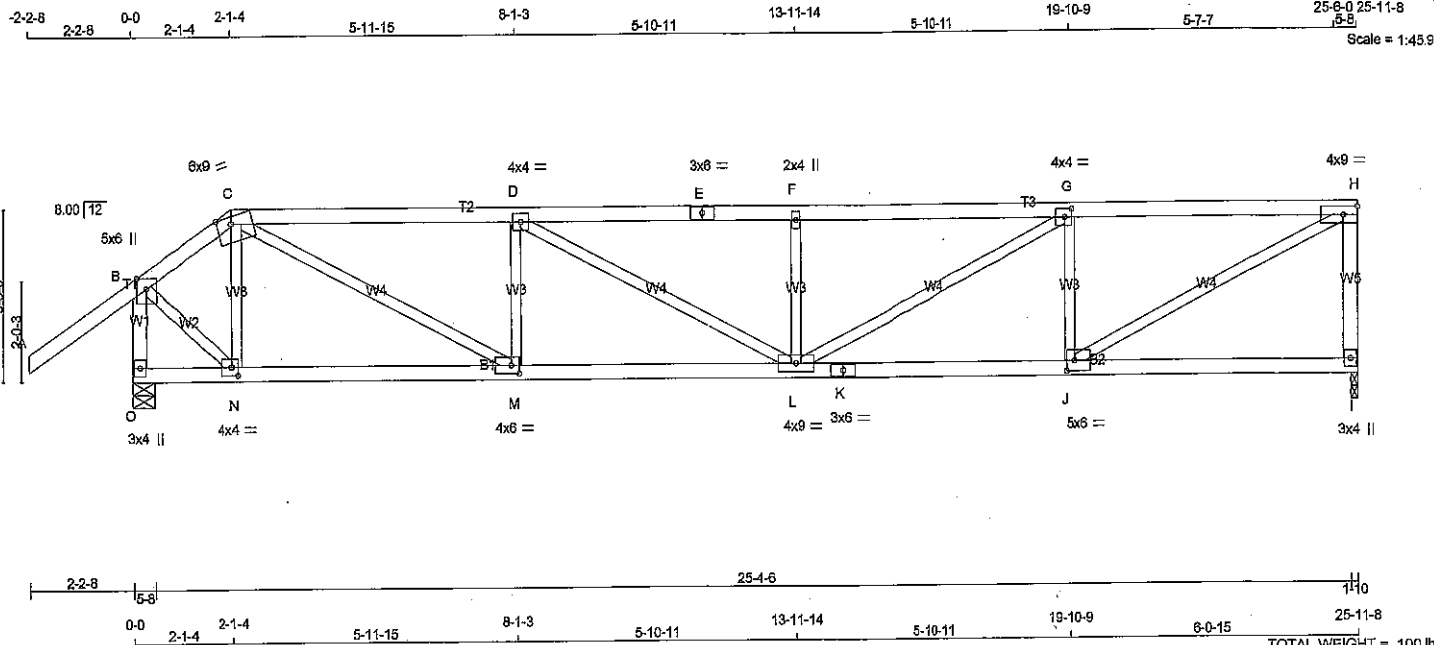
DWG NO. TAM 5118-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286076	T279	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
E - H	2x4	DRY	No.2	SPF	
I - H	2x4	DRY	No.2	SPF	
O - B	2x4	DRY	No.2	SPF	
O - K	2x4	DRY	No.2	SPF	
K - I	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ
JT	1448	0	1448	0
O	1658	0	1858	0

UNFACTORED REACTIONS

	1ST CASE	MAX. MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	1191	659 / 0	273 / 0	0 / 0	0 / 0	259 / 0	0 / 0
O	1333	786 / 0	273 / 0	0 / 0	0 / 0	274 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.38 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 53	-84.3	-84.3	0.31 (1)	10.00	N-C	-528 / 0
B-C	-1127 / 0	-84.3	-84.3	0.32 (1)	5.54	B-N	0 / 1116
C-D	-2664 / 0	-84.3	-84.3	0.83 (1)	3.65	J-H	0 / 2563
D-E	-3051 / 0	-84.3	-84.3	0.68 (1)	3.39	C-M	0 / 2023
E-F	-3051 / 0	-84.3	-84.3	0.68 (1)	3.39	J-G	-984 / 0
F-G	-3051 / 0	-84.3	-84.3	0.89 (1)	3.38	M-D	-788 / 0
G-H	-2279 / 0	-84.3	-84.3	0.59 (1)	3.83	L-G	0 / 874
I-H	-1378 / 0	0.0	0.0	0.25 (1)	6.93	D-L	0 / 438
O-B	-1662 / 0	0.0	0.0	0.19 (1)	8.43	L-F	-457 / 0
O-N	0 / 0	-28.0	-28.0	0.16 (2)	10.00		
N-M	0 / 869	-28.0	-28.0	0.29 (2)	10.00		
M-L	0 / 2864	-28.0	-28.0	0.52 (1)	10.00		
L-K	0 / 2279	-28.0	-28.0	0.50 (2)	10.00		
K-J	0 / 2279	-28.0	-28.0	0.50 (2)	10.00		
J-I	0 / 0	-28.0	-28.0	0.24 (3)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.8 PSF
DL = 3.0 PSF	
BOT CH.	LL = 10.5 PSF
DL = 7.0 PSF	
TOTAL LOAD = 46.1 PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.87")
CALCULATED VERT. DEFL.(LL) = $L/998$ (0.20")
ALLOWABLE DEFL.(TL) = $L/360$ (0.87")
CALCULATED VERT. DEFL.(TL) = $L/952$ (0.33")

CS1: TC=0.69/1.00 (F-G:1), BC=0.52/1.00 (L-M:1),
WB=0.58/1.00 (H-J:1), SS=0.24/1.00 (G-H:1)

DOI, LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.58 (K) (INPUT = 1.00)



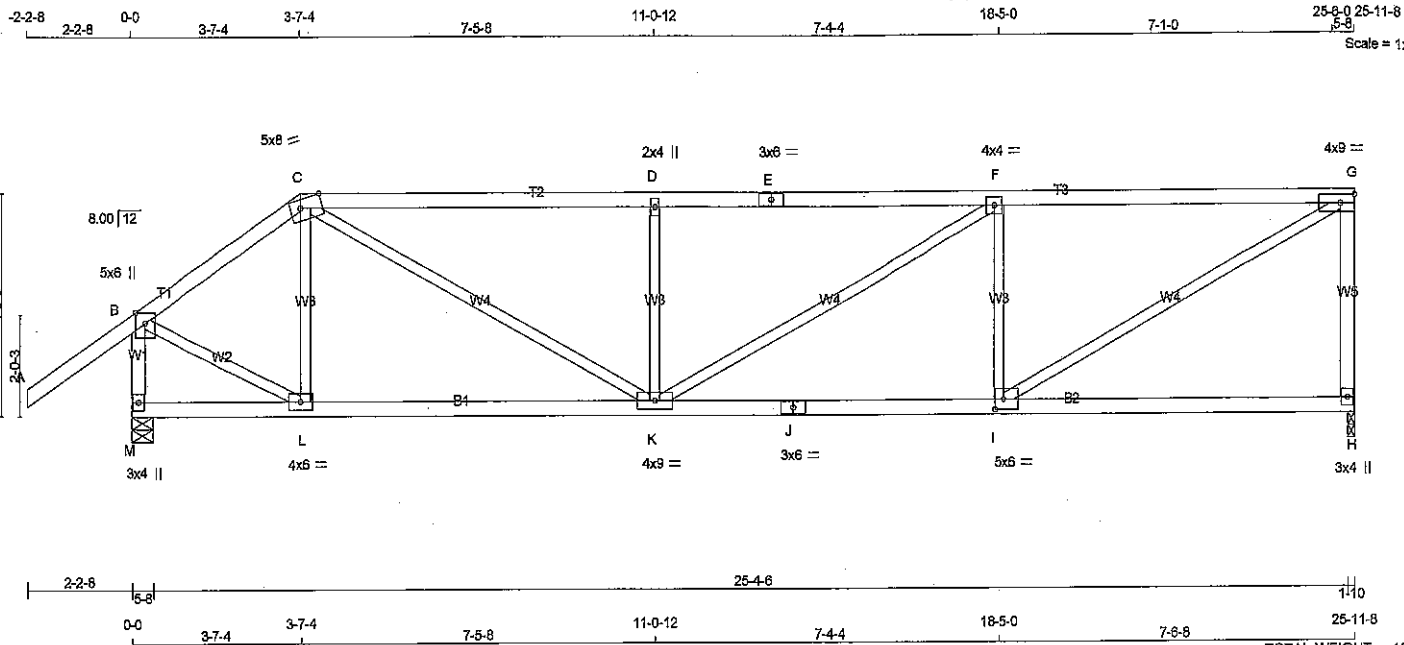
DRWG NO. TAM5119
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286076	T280	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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Scale = 1:45.9

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
H - G	2x4	DRY	No.2	SPF	
M - B	2x4	DRY	No.2	SPF	
M - J	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWV+p	MT20	5.0	6.0	Edge	
C	TTWW-m	MT20	5.0	6.0	Edge	5.50
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWV-t	MT20	4.0	4.0		
G	TMWV-t	MT20	4.0	9.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMWV-t	MT20	5.0	6.0	2.50	2.25
J	BS-t	MT20	3.0	6.0		
K	BMWV-t	MT20	4.0	9.0		
L	BMWV-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edges - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	1457	0	1457	0	0	1-10	1-10
M	1650	0	1650	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST CASE	MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
H	1198	864 / 0	273 / 0	0 / 0	0 / 0	280 / 0	0 / 0
M	1327	781 / 0	273 / 0	0 / 0	0 / 0	273 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.39 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC2)
FR-TO				FR-TO			
A-B	0 / 53	-84.3	-84.3 0.31 (1)	10.00	L-C	-335 / 41	0.10 (1)
B-C	-1369 / 0	-84.3	-84.3 0.22 (1)	5.31	B-L	0 / 1250	0.28 (1)
C-D	-2374 / 0	-84.3	-84.3 0.95 (1)	3.44	I-G	0 / 2321	0.52 (1)
D-E	-2374 / 0	-84.3	-84.3 0.97 (1)	3.39	C-K	0 / 1423	0.32 (1)
E-F	-2374 / 0	-84.3	-84.3 0.97 (1)	3.39	I-F	-879 / 0	0.25 (1)
F-G	-2038 / 0	-84.3	-84.3 0.91 (1)	3.67	K-D	-677 / 0	0.20 (1)
H-G	-1371 / 0	0.0	0.0 0.42 (1)	6.94	K-F	0 / 388	0.09 (1)
M-B	-1828 / 0	0.0	0.0 0.19 (1)	6.48			
M-L	0 / 0	-28.0	-28.0 0.24 (2)	10.00			
L-K	0 / 1129	-28.0	-28.0 0.41 (2)	10.00			
K-J	0 / 2036	-28.0	-28.0 0.61 (2)	10.00			
J-I	0 / 2036	-28.0	-28.0 0.61 (2)	10.00			
I-H	0 / 0	-28.0	-28.0 0.38 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.87")
CALCULATED VERT. DEFL.(LL) = L/999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.87")
CALCULATED VERT. DEFL.(TL) = L/999 (0.25")

CSI: TC=0.97/1.00 (D-F:1), BC=0.61/1.00 (I-K:2),
WB=0.52/1.00 (G-I:1), SSI=0.30/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
MT20	618	354
	1687	822
	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

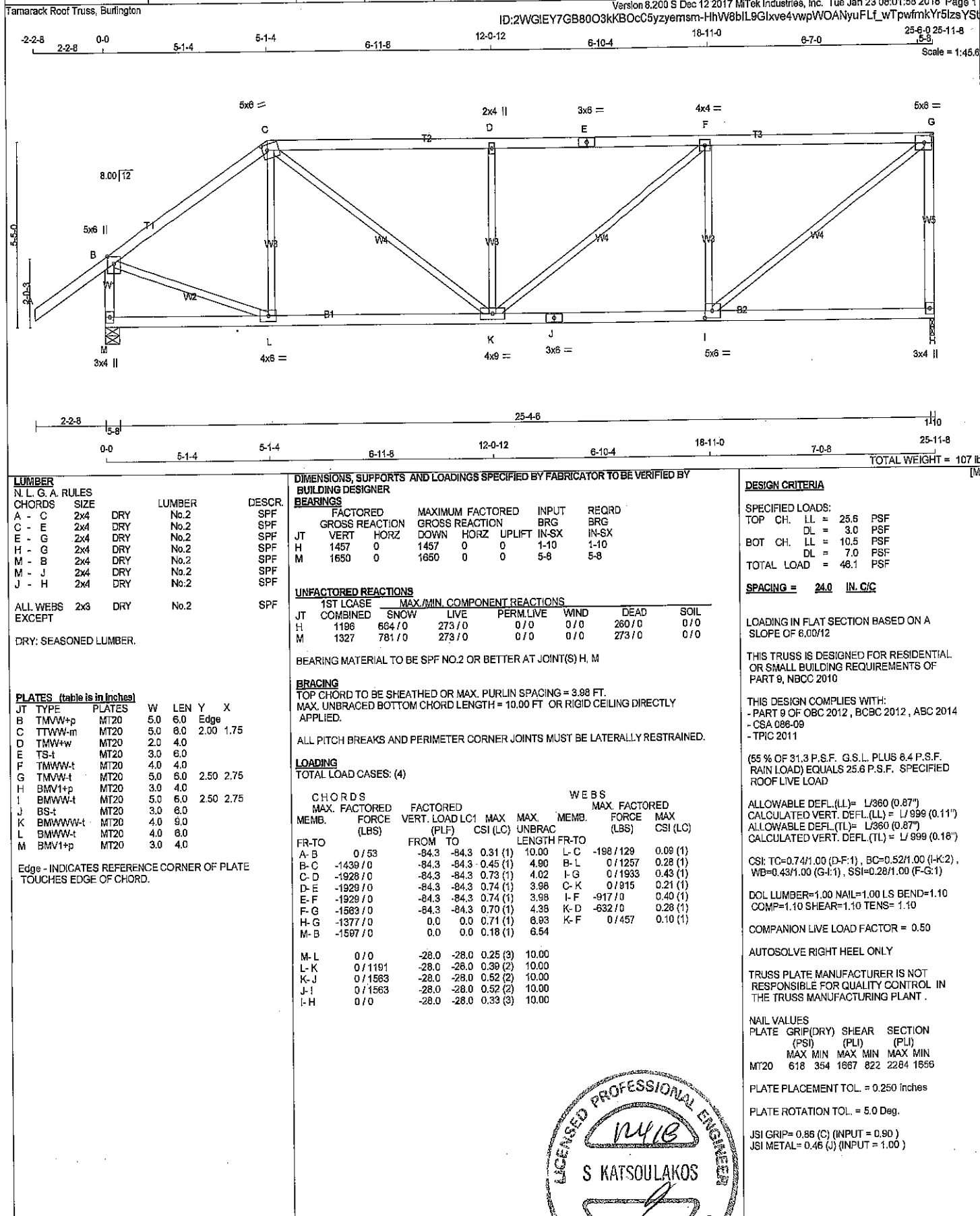
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (G) (INPUT = 0.90)
JSI METAL= 0.84 (J) (INPUT = 1.00)



DRWG NO. TAM 5120-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286076	T281	1	1	TRUSS DESC.	



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - E	2x4	DRY No.2	SPF
E - G	2x4	DRY No.2	SPF
H - G	2x4	DRY No.2	SPF
M - B	2x4	DRY No.2	SPF
M - J	2x4	DRY No.2	SPF
J - H	2x4	DRY No.2	SPF

ALL WEBS	2x3	DRY No.2	SPF
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	5.0	6.0	Edge
C	TTWW-m	MT20	5.0	6.0	2.00 1.75
D	TMVW+w	MT20	2.0	4.0	
E	TS-t	MT20	3.0	6.0	
F	TMVW-t	MT20	4.0	4.0	
G	TMVW-t	MT20	5.0	6.0	2.50 2.75
H	BMV1+p	MT20	3.0	4.0	
I	BMVW-t	MT20	5.0	6.0	2.50 2.75
J	BS-t	MT20	3.0	6.0	
K	BMVWVW-t	MT20	4.0	9.0	
L	BMVW-t	MT20	4.0	6.0	
M	BMV1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
H	1457	0	1457	0	1-10
M	1650	0	1650	0	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN	COMPONENT REACTIONS	WIND	DEAD	SOIL
JT	COMBINED	SNOW	LIVE	PERM.LIVE	UNBRAC	
H	1196	664 / 0	273 / 0	0 / 0	0 / 0	260 / 0
M	1327	781 / 0	273 / 0	0 / 0	0 / 0	273 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX.	MEMB.	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT.	(PLF)	CSI (LC)	UNBRAC	LENGTH	FR-TO	FORCE (LBS)	MAX
FR-TO		FROM	TO						
A-B	0 / 53	-84.3	-84.3	0.31 (1)	10.00	L-C	-198 / 129	0.09 (1)	
B-C	-1439 / 0	-84.3	-84.3	0.45 (1)	4.90	B-L	0 / 1257	0.28 (1)	
C-D	-1928 / 0	-84.3	-84.3	0.73 (1)	4.02	I-G	0 / 1933	0.43 (1)	
D-E	-1929 / 0	-84.3	-84.3	0.74 (1)	3.98	C-K	0 / 915	0.21 (1)	
E-F	-1929 / 0	-84.3	-84.3	0.74 (1)	3.98	I-F	-917 / 0	0.40 (1)	
F-G	-1583 / 0	-84.3	-84.3	0.70 (1)	4.35	K-D	-632 / 0	0.28 (1)	
H-G	-1377 / 0	0.0	0.0	0.71 (1)	6.93	K-F	0 / 457	0.10 (1)	
M-B	-1597 / 0	0.0	0.0	0.18 (1)	6.54				
M-L	0 / 0	-28.0	-28.0	0.25 (3)	10.00				
L-K	0 / 1191	-28.0	-28.0	0.39 (2)	10.00				
K-J	0 / 1583	-28.0	-28.0	0.52 (2)	10.00				
J-I	0 / 1563	-28.0	-28.0	0.52 (2)	10.00				
I-H	0 / 0	-28.0	-28.0	0.33 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.87")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.11")
ALLOWABLE DEFL.(TL) = $L/360$ (0.87")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.18")

CSI: TC=0.74/1.00 (D-F:1), BC=0.52/1.00 (I-K:2),
WB=0.43/1.00 (G-I:1), SSI=0.28/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	618	354	1667
	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

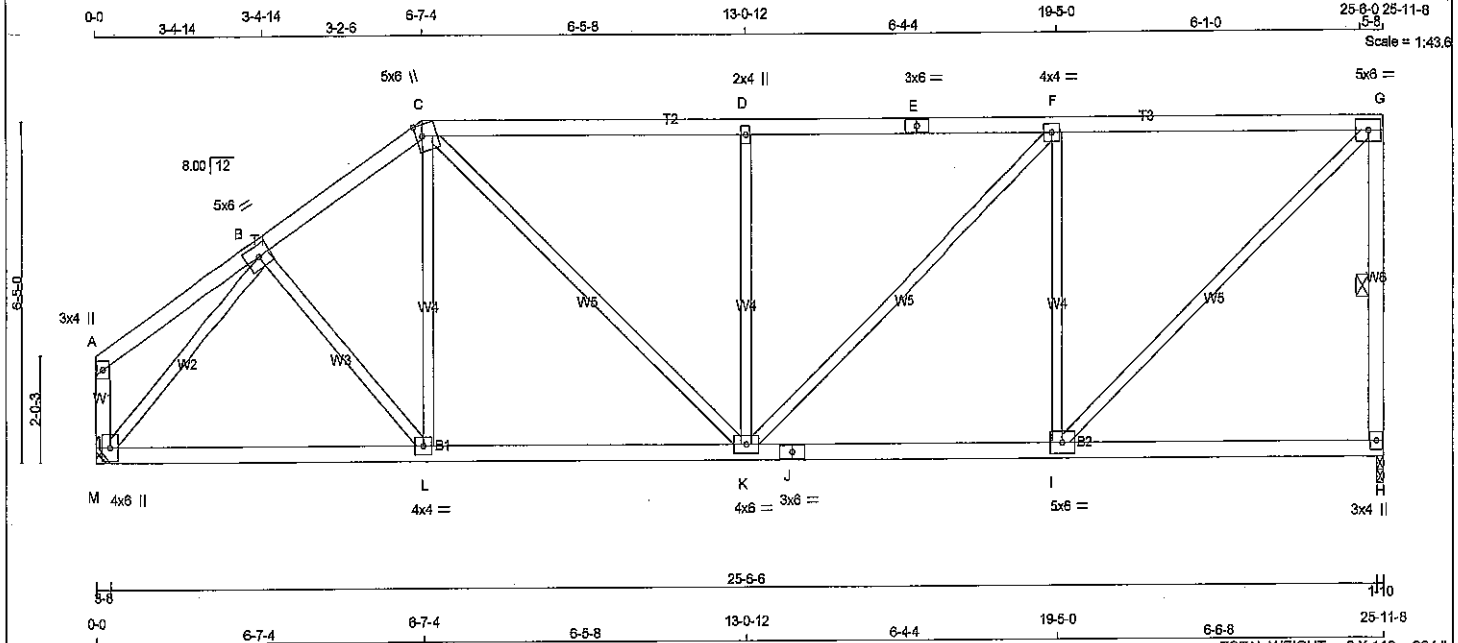
JSI GRIP= 0.85 (C) (INPUT = 0.90)
JSI METAL= 0.46 (J) (INPUT = 1.00)



DRWG NO. TAM 5121-18
STRUCTURAL
COMPONENT ONLY

Tamarack Roof Truss, Burlington

ID:2WGIEY7GB80O3kKB0cC5yzyemsm-mt4WpeMn1c3mGEU7NEvPwARSV3LACCCO?OI0aCzsYSs



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - E	2x4	DRY No.2	SPF
E - G	2x4	DRY No.2	SPF
H - G	2x4	DRY No.2	SPF
M - A	2x4	DRY No.2	SPF
M - J	2x4	DRY No.2	SPF
J - H	2x4	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	5.0	6.0		
C	TTWW+m	MT20	5.0	6.0	2.50	1.50
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMWW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMVW1+p	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT 1457	DOWN 1457	IN-SX 1-10	IN-SX 1-10
H	HORZ 0	HORZ 0	1-10	1-10
M	HORZ 0	HORZ 0	HANGER BY OTHERS	MIN. SEAT SIZE: 3-8

UNFACTORED REACTIONS

1ST LCASE	MAX MIN. COMPONENT REACTIONS
JT COMBINED	SNOW 684 / 0
H	LIVE 273 / 0
M	PERM. LIVE 0 / 0
	WIND 0 / 0
	DEAD 260 / 0
	SOIL 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.49 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		WEBS	
	MAX. FACTORED	FORCE	VERT. LOAD	MAX. FACTORED	MAX. FACTORED	FORCE
FR-TO		(LBS)	FROM TO	CS1 (LC)	UNBRAC LENGTH	FR-TO
A-B	0 / 16	-84.3	-84.3	0.13 (1)	10.00	B-L
B-C	-1455 / 0	-84.3	-84.3	0.16 (1)	5.26	L-C
C-D	-1609 / 0	-84.3	-84.3	0.57 (1)	4.52	M-B
D-E	-1609 / 0	-84.3	-84.3	0.59 (1)	4.49	I-G
E-F	-1809 / 0	-84.3	-84.3	0.59 (1)	4.49	C-K
F-G	-1240 / 0	-84.3	-84.3	0.55 (1)	5.01	I-F
H-G	-1384 / 0	0.0	0.0	0.26 (1)	5.53	K-D
M-A	-109 / 0	0.0	0.0	0.01 (1)	7.81	K-F
M-L	0 / 1077	-28.0	-28.0	0.43 (2)	10.00	
L-K	0 / 1196	-28.0	-28.0	0.45 (2)	10.00	
K-J	0 / 1240	-28.0	-28.0	0.44 (2)	10.00	
J-I	0 / 1240	-28.0	-28.0	0.44 (2)	10.00	
I-H	0 / 0	-28.0	-28.0	0.30 (3)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.87")
CALCULATED VERT. DEFL.(LL) = L/999 (0.09")
ALLOWABLE DEFL.(TL) = L/360 (0.87")
CALCULATED VERT. DEFL.(TL) = L/999 (0.14")

CS1: TC=0.59/1.00 (D-F:1), BC=0.45/1.00 (K-L:2), WB=0.69/1.00 (B-M:1), SS=0.26/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

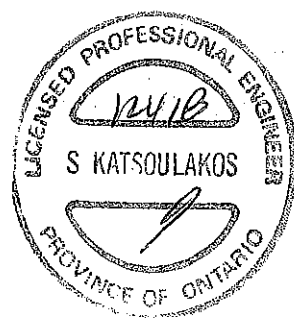
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	616 354	1667 822	2284 1656

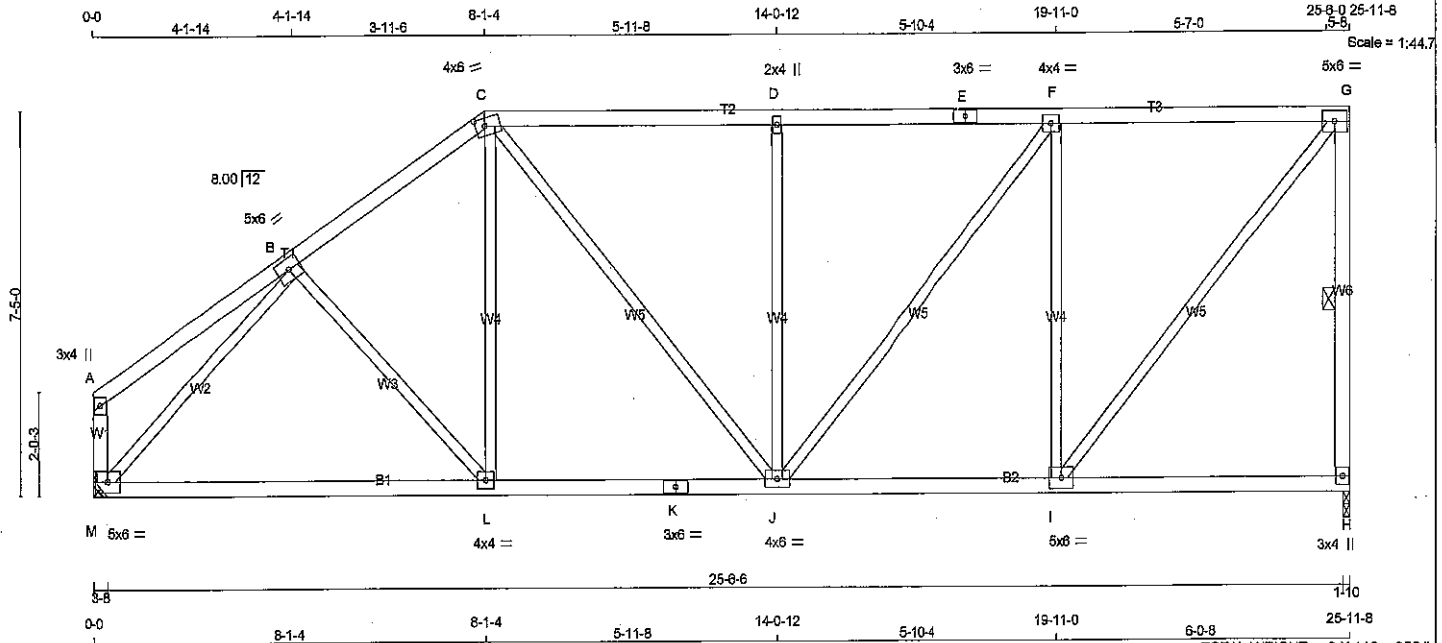
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (M) (INPUT = 0.90)
JSI METAL= 0.41 (B) (INPUT = 1.00)



DWG NO. TAM 5122-18
STRUCTURAL
COMPONENT ONLY



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - E	2x4	DRY No.2	SPF
E - G	2x4	DRY No.2	SPF
H - G	2x4	DRY No.2	SPF
M - A	2x4	DRY No.2	SPF
M - K	2x4	DRY No.2	SPF
K - H	2x4	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	5.0	6.0		
C	TTWW-m	MT20	4.0	6.0	1.75	2.25
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	5.0	6.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BS-t	MT20	3.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMVW-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT VERT	1457	0	1-10	1-10
H	1457	0	1-10	1-10
M	1457	0	1-10	1-10

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
H	1196	864 / 0	273 / 0	0 / 0	0 / 0	260 / 0	0 / 0
M	1196	864 / 0	273 / 0	0 / 0	0 / 0	260 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 21	-84.3 -84.3	0.21 (1)	10.00	B-L	0 / 151	0.03 (3)
B-C	-1424 / 0	-84.3 -84.3	0.25 (1)	5.19	L-C	0 / 321	0.07 (2)
C-D	-1383 / 0	-84.3 -84.3	0.46 (1)	4.99	M-B	-1674 / 0	1.00 (1)
D-E	-1383 / 0	-84.3 -84.3	0.47 (1)	4.96	I-G	0 / 1553	0.35 (1)
E-F	-1363 / 0	-84.3 -84.3	0.47 (1)	4.98	C-J	0 / 304	0.07 (1)
F-G	-1004 / 0	-84.3 -84.3	0.45 (1)	5.60	I-F	-989 / 0	0.97 (1)
H-G	-1390 / 0	0.0 0.0	0.34 (1)	5.52	J-D	-540 / 0	0.53 (1)
M-A	-132 / 0	0.0 0.0	0.02 (1)	7.81	J-F	0 / 585	0.13 (1)
M-L	0 / 1134	-28.0 -28.0	0.54 (2)	10.00			
L-K	0 / 1168	-28.0 -28.0	0.56 (2)	10.00			
K-J	0 / 1168	-28.0 -28.0	0.56 (2)	10.00			
J-I	0 / 1004	-28.0 -28.0	0.38 (2)	10.00			
I-H	0 / 0	-28.0 -28.0	0.26 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.87")
CALCULATED VERT. DEFL.(LL) = L/999 (0.18")
ALLOWABLE DEFL.(TL) = L/380 (0.87")
CALCULATED VERT. DEFL.(TL) = L/999 (0.30")

CSI: TO=0.47/1.00 (D-F:1), BC=0.56/1.00 (J-L:2), WB=1.00/1.00 (B-M:1), SS=0.24/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1837 822	2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.41 (B) (INPUT = 1.00)



DWG NO. TAM 5123-18
STRUCTURAL
COMPONENT ONLY

JOB NAME 283746	TRUSS NAME T284	QUANTITY 1	PLY 1	JOB DESC. 43954	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Mon Sep 11 14:28:43 2017 Page 1	
ID:2WGIEY7GB8003kKBQcC5zyemsm-wT44CBPon2U_rMRcBkvxtQ1wpgKSZ5opCH247OyehYY					

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
A - C	2x6	DRY	No.2
C - E	2x6	DRY	No.2
E - G	2x6	DRY	No.2
H - G	2x6	DRY	No.2
M - B	2x6	DRY	No.2
M - J	2x6	DRY	No.2
J - H	2x6	DRY	No.2

ALL WEBS 2x4 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.50	3.75
C	TTWW+m	MT20	8.0	8.0	Edge	6.50
D	TMW-w	MT20	3.0	6.0		
E	TS-t	MT20	5.0	6.0		
F	TMVWV-t	MT20	5.0	12.0	2.50	2.75
G	TMV-p	MT20	3.0	6.0		
H	BMVW1-t	MT20	6.0	9.0	3.00	4.25
I	BMVW-w	MT20	3.0	6.0		
J	BS-t	MT20	5.0	6.0		
K	BMVWV-t	MT20	5.0	12.0		
L	BMVW-t	MT20	5.0	6.0	2.50	2.00
M	BMV1-p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 104.0 lbs FACTORED DOWN AT 12-0-12, 104.0 lbs FACTORED DOWN AT 14-0-12, 104.0 lbs FACTORED DOWN AT 16-0-12, 104.0 lbs FACTORED DOWN AT 18-0-12, 104.0 lbs FACTORED DOWN AT 20-0-12, 104.0 lbs FACTORED DOWN AT 22-0-12, AND 104.0 lbs FACTORED DOWN AT 24-0-12, AND 120.7 lbs FACTORED DOWN AT 25-6-12 ON TOP CHORD, AND 985.4 lbs FACTORED DOWN AT 10-1-8, 71.5 lbs FACTORED DOWN AT 12-0-12, 71.5 lbs FACTORED DOWN AT 14-0-12, 71.5 lbs FACTORED DOWN AT 16-0-12, 71.5 lbs FACTORED DOWN AT 18-0-12, 71.5 lbs FACTORED DOWN AT 20-0-12, 71.5 lbs FACTORED DOWN AT 22-0-12, AND 71.5 lbs FACTORED DOWN AT 24-0-12, AND 81.2 lbs FACTORED DOWN AT 25-6-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
H	2705	2705	0	0
M	2498	2498	0	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
H	2226	1228 / 0	512 / 0	0 / 0	486 / 0	0 / 0
M	2022	1169 / 0	429 / 0	0 / 0	425 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2 LATERAL BRACE(S) AT 1/3 LENGTH OF F-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)
FR-TO									
A-B	0 / 33	-84.3	-84.3	0.07 (1)	10.00	L-C	-119 / 138	0.02 (3)	0.46 (1)
B-C	-3048 / 0	-84.3	-84.3	0.19 (1)	4.64	B-L	0 / 2585	0.82 (1)	0.54 (1)
C-D	-5281 / 0	-84.3	-84.3	0.56 (1)	3.34	F-H	-4642 / 0	0.12 (3)	0.14 (1)
D-N	-5281 / 0	-84.3	-84.3	0.84 (1)	2.89	C-K	0 / 3080	0.12 (3)	0.14 (1)
N-O	-5281 / 0	-84.3	-84.3	0.84 (1)	2.89	I-F	0 / 688	0.14 (1)	0.22 (1)
O-E	-5281 / 0	-84.3	-84.3	0.84 (1)	2.89	K-D	-813 / 0	0.14 (1)	0.22 (1)
E-P	-5281 / 0	-84.3	-84.3	0.84 (1)	2.89	K-F	0 / 1252	0.14 (1)	0.22 (1)
P-F	-5281 / 0	-84.3	-84.3	0.84 (1)	2.89				
F-Q	0 / 0	-84.3	-84.3	0.54 (1)	10.00				
Q-R	0 / 0	-84.3	-84.3	0.54 (1)	10.00				
R-S	0 / 0	-84.3	-84.3	0.54 (1)	10.00				
S-T	0 / 0	-84.3	-84.3	0.54 (1)	10.00				
T-G	0 / 0	-84.3	-84.3	0.54 (1)	10.00				
H-G	-471 / 0	0.0	0.0	0.07 (1)	7.81				
M-B	-2525 / 0	0.0	0.0	0.18 (1)	6.48				
M-L	0 / 0	-28.0	-28.0	0.27 (1)	10.00				
L-U	0 / 2524	-28.0	-28.0	0.85 (1)	10.00				
U-K	0 / 2524	-28.0	-28.0	0.85 (1)	10.00				
K-V	0 / 4166	-28.0	-28.0	0.89 (1)	10.00				
V-J	0 / 4166	-28.0	-28.0	0.89 (1)	10.00				
J-W	0 / 4166	-28.0	-28.0	0.89 (1)	10.00				
W-X	0 / 4166	-28.0	-28.0	0.89 (1)	10.00				
X-I	0 / 4166	-28.0	-28.0	0.89 (1)	10.00				
I-Y	0 / 4166	-28.0	-28.0	0.76 (1)	10.00				
Y-Z	0 / 4166	-28.0	-28.0	0.76 (1)	10.00				
Z-AA	0 / 4166	-28.0	-28.0	0.76 (1)	10.00				
AA-AB	0 / 4166	-28.0	-28.0	0.76 (1)	10.00				
AB-H	0 / 4166	-28.0	-28.0	0.76 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 3.0 PSF
BOT CH. LL	= 10.5 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = 1/360 (0.87")
CALCULATED VERT. DEFL. (LL) = 1/999 (0.24")
ALLOWABLE DEFL. (TL) = 1/360 (0.87")
CALCULATED VERT. DEFL. (TL) = 1/822 (0.38")

CSI: TC=0.84 (D-F:1), BC=0.89 (I-K:1), WB=0.82 (F-H:1), SSI=0.80 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

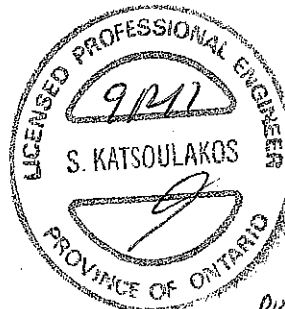
JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.89 (H) (INPUT = 1.00)



DRWG NO. TAM46129-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
283746	T284	1	1	TRUSS DESC.		
Tamarack Roof Truss, Burlington						
Version 8.030 S Oct 5 2016 MITek Industries, Inc. Mon Sep 11 14:28:44 2017 Page 2						
ID:2WGIEY7GB80O3kKB0cC5zyemsm-OfeSPXQ8MdrTW0plSQAQda5Z4fhly2yRxodfryehYX						

FACTORED CONCENTRATED LOADS (LBS)						
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.
E	16-0-12	-104	-104	---	BACK	VERT
J	14-0-12	-41	-71	---	BACK	VERT
N	12-0-12	-104	-104	---	BACK	VERT
O	14-0-12	-104	-104	---	BACK	VERT
P	18-0-12	-104	-104	---	BACK	VERT
Q	20-0-12	-104	-104	---	BACK	VERT
R	22-0-12	-104	-104	---	BACK	VERT
S	24-0-12	-104	-104	---	BACK	VERT
T	25-6-12	-121	-121	---	BACK	VERT
U	10-1-8	-965	-965	---	BACK	VERT
V	12-0-12	-41	-71	---	BACK	VERT
W	16-0-12	-41	-71	---	BACK	VERT
X	18-0-12	-41	-71	---	BACK	VERT
Y	20-0-12	-41	-71	---	BACK	VERT
Z	22-0-12	-41	-71	---	BACK	VERT
AA	24-0-12	-41	-71	---	BACK	VERT
AB	25-6-12	-48	-81	---	BACK	VERT
						TOTAL



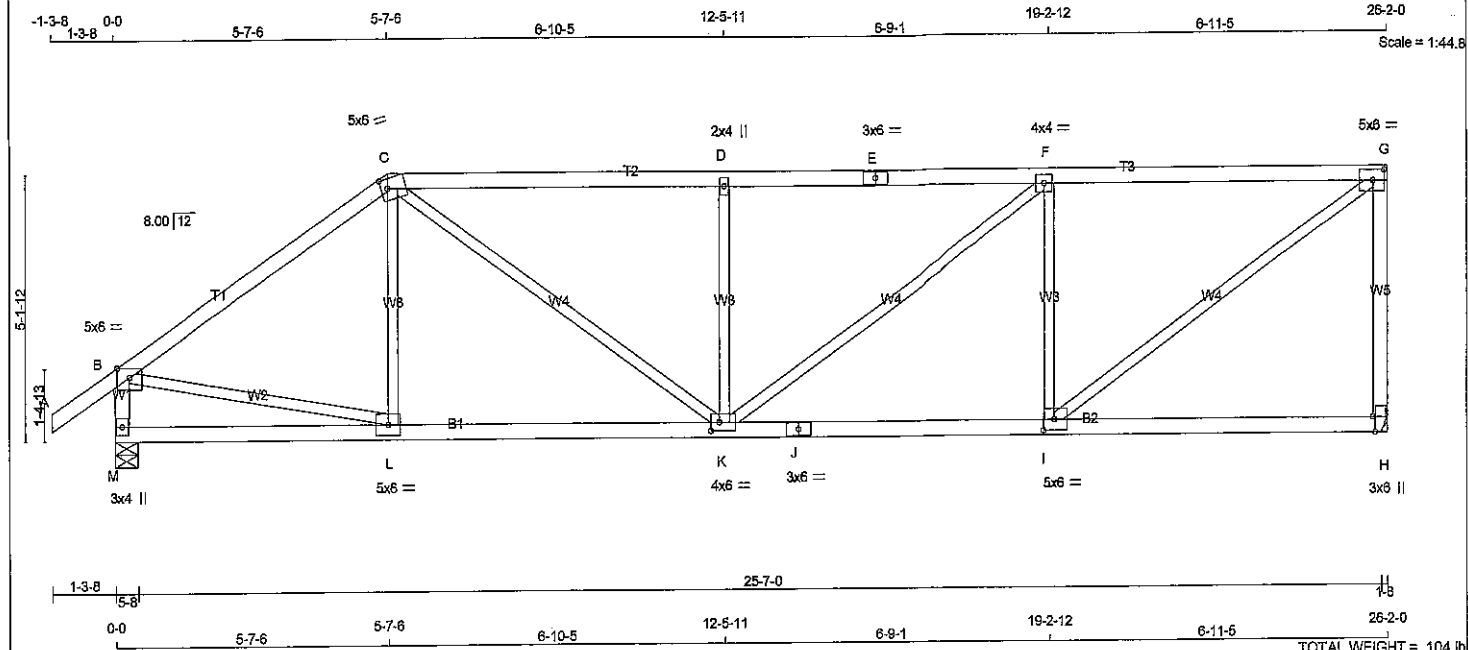
DWG NO. TAM 46129-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
283746	T285	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Mon Sep 11 14:28:44 2017 Page 1

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TOTAL WEIGHT = 104 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4 DRY	No.2	SPF		
C - E	2x4 DRY	No.2	SPF		
E - G	2x4 DRY	No.2	SPF		
H - G	2x4 DRY	No.2	SPF		
M - B	2x4 DRY	No.2	SPF		
J - J	2x4 DRY	No.2	SPF		
J - H	2x4 DRY	No.2	SPF		

ALL WEBS 2x3 DRY No.2 SPF EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TTWW-w	MT20	5.0	6.0	2.25	1.50
D	TMVW-w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMVW-t	MT20	4.0	4.0		
G	TMVW-t	MT20	5.0	6.0	2.50	2.75
H	BMV1-t	MT20	3.0	6.0	Edge	0.50
I	BMVW-t	MT20	5.0	6.0	2.50	2.75
J	BS-t	MT20	3.0	6.0		
K	BMVW-t	MT20	4.0	6.0	2.00	2.00
L	BMVW-t	MT20	5.0	6.0		
M	BMV1-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
H	1489	0	1489	0	0	0
M	1584	0	1584	0	0	0

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
H	1206	669 / 0	275 / 0	0 / 0	0 / 0	262 / 0	0 / 0
M	1284	740 / 0	275 / 0	0 / 0	0 / 0	270 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO			FR-TO			
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	L-C	-60 / 209	0.05 (3)
B-C	-1842 / 0	-84.3	-84.3	0.58 (1)	4.49	B-L	0 / 1389	0.31 (1)
C-D	-2069 / 0	-84.3	-84.3	0.72 (1)	3.92	I-G	0 / 2013	0.45 (1)
D-E	-2069 / 0	-84.3	-84.3	0.74 (1)	3.88	C-K	0 / 864	0.19 (1)
E-F	-2069 / 0	-84.3	-84.3	0.74 (1)	3.88	I-F	-936 / 0	0.37 (1)
F-G	-1649 / 0	-84.3	-84.3	0.68 (1)	4.32	K-D	-622 / 0	0.24 (1)
H-G	-1390 / 0	0.0	0.0	0.63 (1)	6.90	K-F	0 / 517	0.12 (1)
M-B	-1523 / 0	0.0	0.0	0.16 (1)	8.68			
M-L	0 / 0	-28.0	-28.0	0.28 (3)	10.00			
L-K	0 / 1363	-28.0	-28.0	0.43 (2)	10.00			
K-J	0 / 1649	-28.0	-28.0	0.52 (2)	10.00			
J-I	0 / 1649	-28.0	-28.0	0.52 (2)	10.00			
I-H	0 / 0	-28.0	-28.0	0.33 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
DL = 3.0 PSF	
BOT CH.	LL = 10.5 PSF
DL = 7.0 PSF	
TOTAL LOAD	= 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.87")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL) = L/360 (0.87")
CALCULATED VERT. DEFL.(TL) = L/999 (0.18")

CSI: TC=0.74 (D-F:1), BC=0.52 (H-K:2), WB=0.45 (G-I:1), SS=0.27 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1987
	822	2284	1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.45 (J) (INPUT = 1.00)

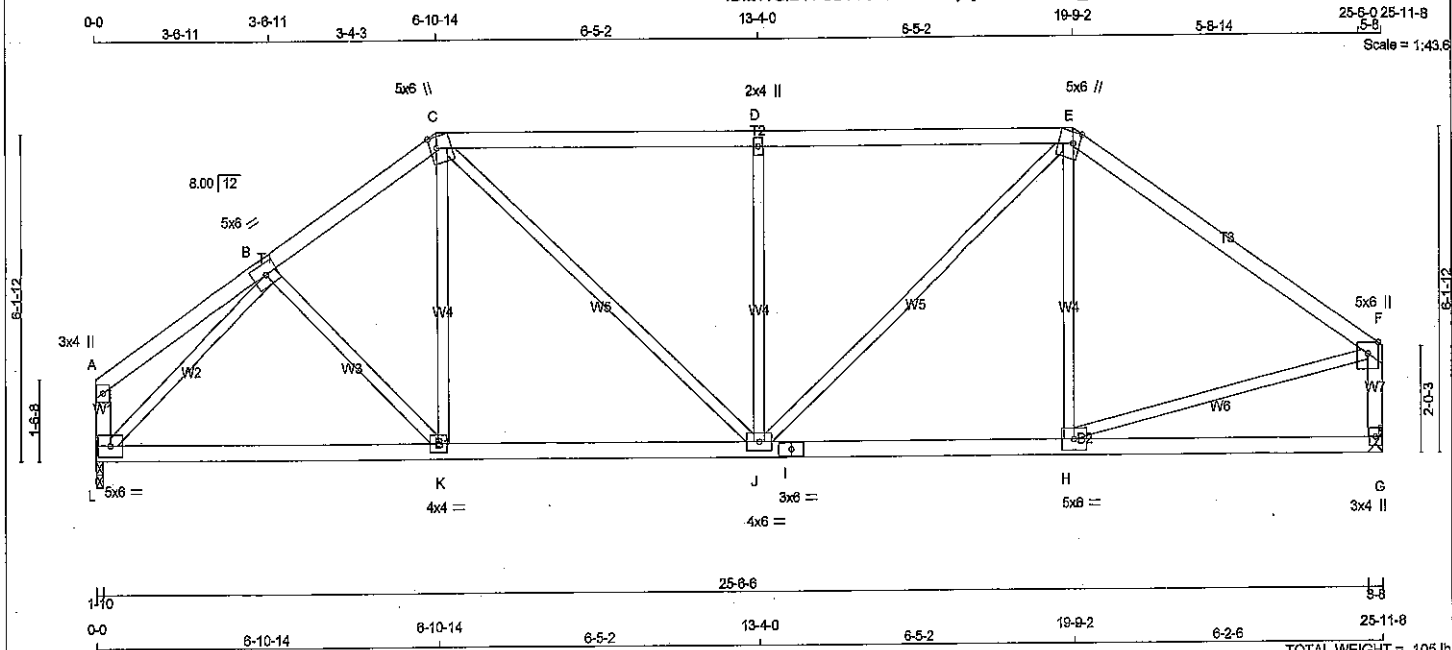


DRWG NO. TAM 46130-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286076	T286A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 105 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
E - F	2x4	DRY	No.2	SPF	
L - A	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
L - I	2x4	DRY	No.2	SPF	
I - G	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0			
B	TMWW-t	MT20	5.0	6.0			
C	TTWW+m	MT20	5.0	6.0	2.50	1.50	
D	TMWW-t	MT20	2.0	4.0			
E	TTWW+m	MT20	5.0	6.0	2.50	1.50	
F	TMWW-t	MT20	5.0	6.0			Edge
G	BMV1+p	MT20	3.0	4.0			
H	BMWW-t	MT20	5.0	6.0			
I	BS-t	MT20	3.0	6.0			
J	BMWW-t	MT20	4.0	6.0			
K	BMWW-t	MT20	4.0	4.0			
L	BMWW-t	MT20	5.0	6.0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
L	1457	0	1457	0	1-10	1-10
G	1457	0	1457	0	0	0

HANGER BY OTHERS
MIN. SEAT SIZE: 3-8

UNFACTORED REACTIONS

JT	1ST CASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
L	1196	664/0	273/0	0/0	0/0	260/0	0/0
G	1196	664/0	273/0	0/0	0/0	260/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.15 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS					
MEMB.	MAX. FACTORED		FACTORED		MAX.	MEMB.	MAX. FACTORED		MAX.		
	FORCE (LBS)	VERT. LOAD (LBS)	LOAD LC1 (PLF)	MAX CSI (LC)			UNBRAC	FORCE (LBS)			MAX CSI (LC)
FR-TO			FROM	TO	LENGTH	FR-TO					
A-B	0/17		-84.3	-84.3	0.14 (1)	10.00	B-K	0/147	0.03 (3)		
B-C	-1566/0		-84.3	-84.3	0.18 (1)	5.09	K-C	-0/278	0.06 (2)		
C-D	-1697/0		-84.3	-84.3	0.71 (1)	4.15	C-J	0/550	0.12 (1)		
D-E	-1697/0		-84.3	-84.3	0.71 (1)	4.15	J-D	-684/0	0.40 (1)		
E-F	-1445/0		-84.3	-84.3	0.70 (1)	4.52	J-E	0/673	0.15 (1)		
L-A	-115/0		0.0	0.0	0.01 (1)	7.81	H-E	-119/188	0.07 (1)		
G-F	-1387/0		0.0	0.0	0.16 (1)	6.90	L-B	-1768/0	0.68 (1)		
							H-F	0/1244	0.28 (1)		
L-K	0/1237		-28.0	-28.0	0.46 (2)	10.00					
K-J	0/1290		-28.0	-28.0	0.48 (2)	10.00					
J-I	0/1199		-28.0	-28.0	0.43 (2)	10.00					
I-H	0/1199		-28.0	-28.0	0.43 (2)	10.00					
H-G	0/0		-28.0	-28.0	0.28 (3)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/380 (0.87")
CALCULATED VERT. DEFL.(LL)= L/999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.87")
CALCULATED VERT. DEFL.(TL)= L/999 (0.17")

CSI: TC=0.71/1.00 (C-D:1), BC=0.48/1.00 (J-K:2), WB=0.68/1.00 (B-L:1), SS=0.28/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2264 1658

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.88 (F) (INPUT = 0.90)
JSI METAL= 0.44 (B) (INPUT = 1.00)



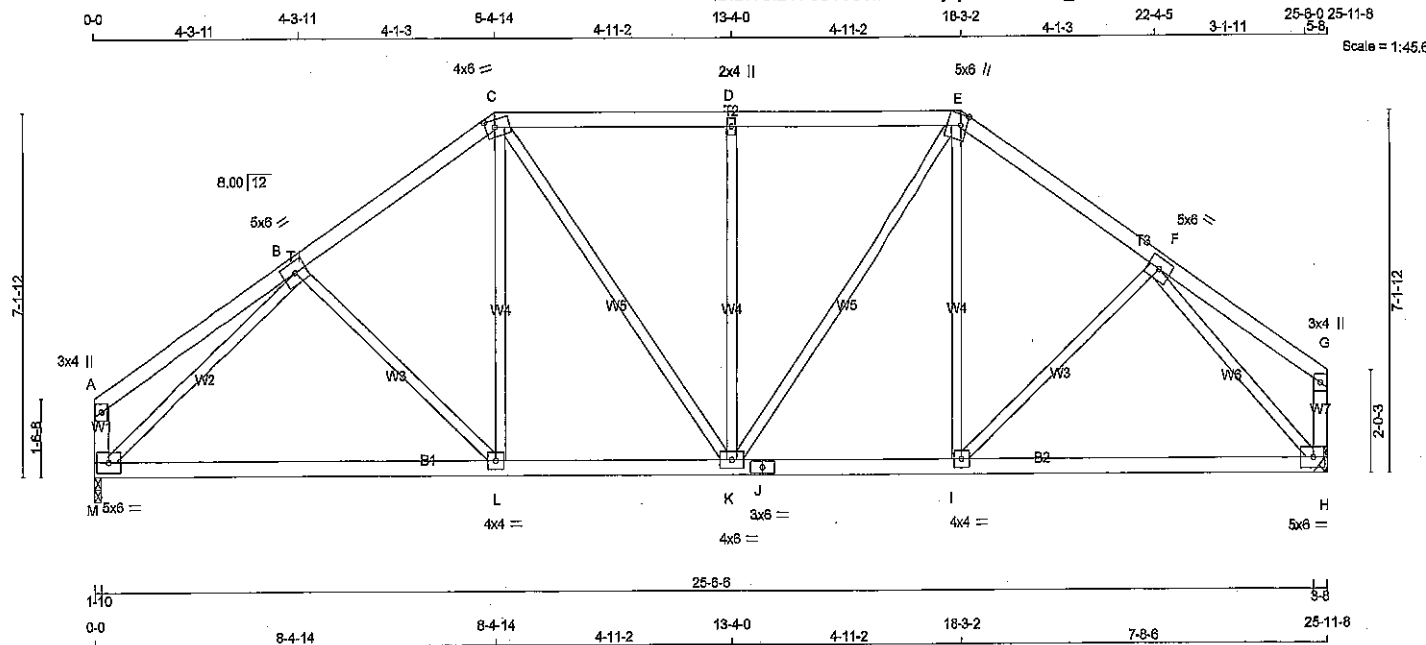
DWG NO. TAM 5126-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286076	T287A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:2WGIEY7GB80O3kKB0c5zyemsm-E4eu0_MPowBduN3JwxQeTNzi0Tf5xahyE21xAezsYSr



LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - A	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY, SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
A	TMV+p	MT20	3.0	4.0
B	TMWW-t	MT20	5.0	6.0
C	TTWW-m	MT20	4.0	6.0 1.75 2.25
D	TMW+w	MT20	2.0	4.0
E	TTWW+m	MT20	5.0	6.0 2.50 1.50
F	TMWW-t	MT20	5.0	6.0
G	TMV+p	MT20	3.0	4.0
H	BMVW1-t	MT20	5.0	6.0
I	BMWW-t	MT20	4.0	4.0
J	BS-t	MT20	3.0	6.0
K	BMVWW-t	MT20	4.0	6.0
L	BMWW-t	MT20	4.0	4.0
M	BMVW1-t	MT20	5.0	6.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG.	REQRD BRG.
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
M	1457	0	1457	0	1-10
H	1457	0	1457	0	1-10

UNFACTORED REACTIONS

1ST LCASE	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	1196	664/0	273/0	0/0	0/0	260/0	0/0
M	1196	664/0	273/0	0/0	0/0	260/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.15 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. MEMB. FORCE (LBS)
FR-TO		FROM TO	FR-TO
A-B	0/22	-84.3 -84.3 0.23 (1)	B-L -57/111 0.03 (1)
B-C	-1509/0	-84.3 -84.3 0.21 (1)	L-C 0/387 0.08 (2)
C-D	-1412/0	-84.3 -84.3 0.28 (1)	C-K 0/295 0.07 (1)
D-E	-1412/0	-84.3 -84.3 0.28 (1)	K-D -505/0 0.45 (1)
E-F	-1434/0	-84.3 -84.3 0.19 (1)	D-E 0/401 0.09 (1)
F-G	0/23	-84.3 -84.3 0.20 (1)	E-F 0/287 0.08 (3)
M-A	-137/0	0.0 0.0 0.01 (1)	F-H 0/179 0.04 (2)
H-G	-108/0	0.0 0.0 0.01 (1)	M-B -1766/0 0.99 (1)
			F-H -1671/0 0.76 (1)
M-L	0/1277	-28.0 -28.0 0.59 (2)	
L-K	0/1240	-28.0 -28.0 0.60 (2)	
K-J	0/1178	-28.0 -28.0 0.52 (2)	
J-I	0/1178	-28.0 -28.0 0.52 (2)	
I-H	0/1095	-28.0 -28.0 0.50 (2)	

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.87")
CALCULATED VERT. DEFL.(LL) = $L/989$ (0.21")
ALLOWABLE DEFL.(TL) = $L/360$ (0.87")
CALCULATED VERT. DEFL.(TL) = $L/800$ (0.35")

CSI: TC=0.28/1.00 (C-D:1), BC=0.60/1.00 (K-L:2),
WB=0.99/1.00 (B-M:1), SS=0.20/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 622 2284 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (C) (INPUT = 0.90)
JSI METAL= 0.43 (B) (INPUT = 1.00)



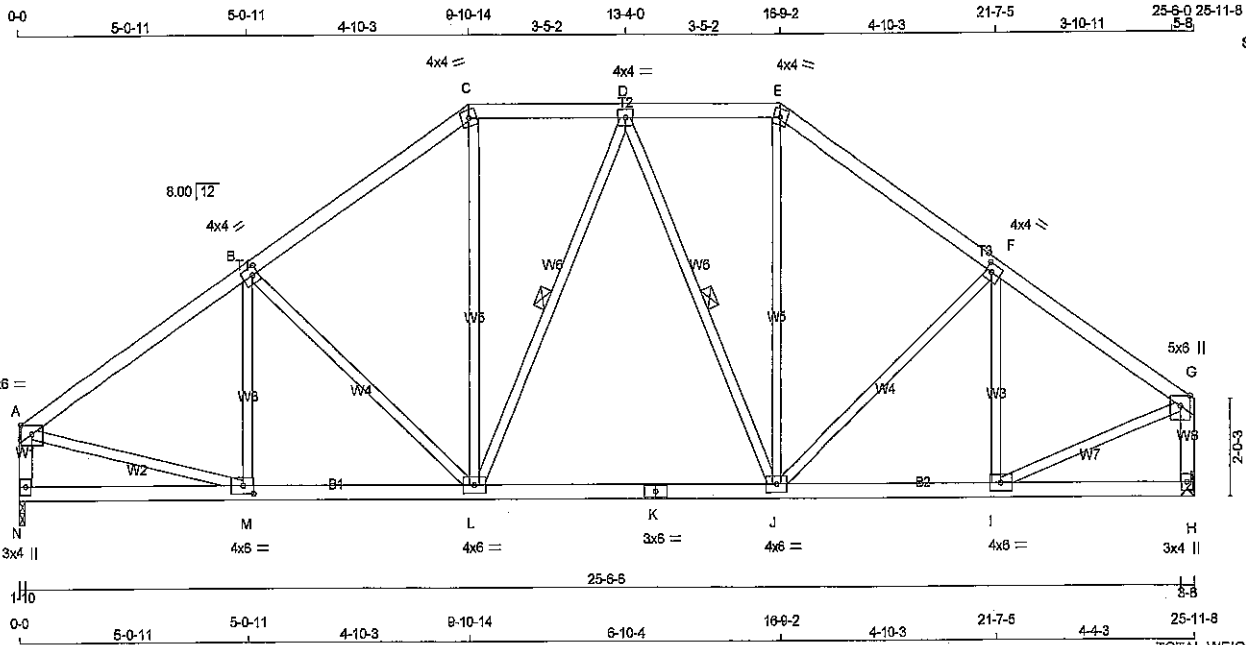
DRWG NO. TAM 5127
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286076	T288A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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Scale: 1/4"=1'

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
N - A	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	6.0	Edge	
B	TMVW-t	MT20	4.0	4.0	2.00	1.50
C	TTW-m	MT20	4.0	4.0		
D	TMVW-t	MT20	4.0	4.0		
E	TTW-m	MT20	4.0	4.0		
F	TMVW-t	MT20	4.0	4.0	2.00	1.50
G	TMVW+p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMVW-t	MT20	4.0	6.0		
J	BMVW-t	MT20	4.0	6.0		
K	BS-t	MT20	3.0	6.0		
L	BMVW-t	MT20	4.0	6.0		
M	BMVW-t	MT20	4.0	6.0	2.00	2.75
N	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
N	1457	0	1457	0	1-10
H	1457	0	1457	0	1-10

UNFACTORED REACTIONS		1ST LCASE	MAX / MIN COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
N	1196	664 / 0	273 / 0
H	1196	664 / 0	273 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.92 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-L, D-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING		CHORDS	WEBS
MEMB.	FORCE (LBS)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED MAX. (LC)

FR-TO	FORCE (LBS)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED MAX. (LC)
A-B	-1804 / 0	-84.3 -84.3 0.30 (1)	4.92
B-C	-1407 / 0	-84.3 -84.3 0.29 (1)	5.19
C-D	-1152 / 0	-84.3 -84.3 0.13 (1)	5.80
D-E	-1119 / 0	-84.3 -84.3 0.13 (1)	5.87
E-F	-1363 / 0	-84.3 -84.3 0.28 (1)	5.29
F-G	-1420 / 0	-84.3 -84.3 0.25 (1)	5.22
N-A	-1396 / 0	0.0 0.0 0.15 (1)	6.69
H-G	-1406 / 0	0.0 0.0 0.16 (1)	6.67
N-M	0 / 0	-28.0 -28.0 0.15 (2)	10.00
M-L	0 / 1357	-28.0 -28.0 0.36 (2)	10.00
L-K	0 / 1208	-28.0 -28.0 0.35 (2)	10.00
K-J	0 / 1208	-28.0 -28.0 0.35 (2)	10.00
J-I	0 / 1204	-28.0 -28.0 0.35 (2)	10.00
I-H	0 / 0	-28.0 -28.0 0.12 (3)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:	
TOP CH.	LL = 25.6 PSF
BOT CH.	LL = 3.0 PSF
	DL = 10.5 PSF
	DL = 7.0 PSF
TOTAL LOAD	= 46.1 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.87")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (0.87")
CALCULATED VERT. DEFL.(TL) = L/999 (0.13")

CSI: TC=0.30/1.00 (A-B:1), BC=0.36/1.00 (L-M:2), WB=0.31/1.00 (A-M:1), SS=0.17/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

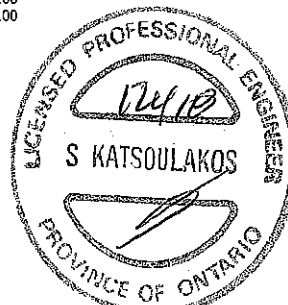
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	
PLATE GRIP(DRY)	SHEAR SECTION (PSI) (PLI) (PLI)
MT20	618 354 1667 822 2284 1856

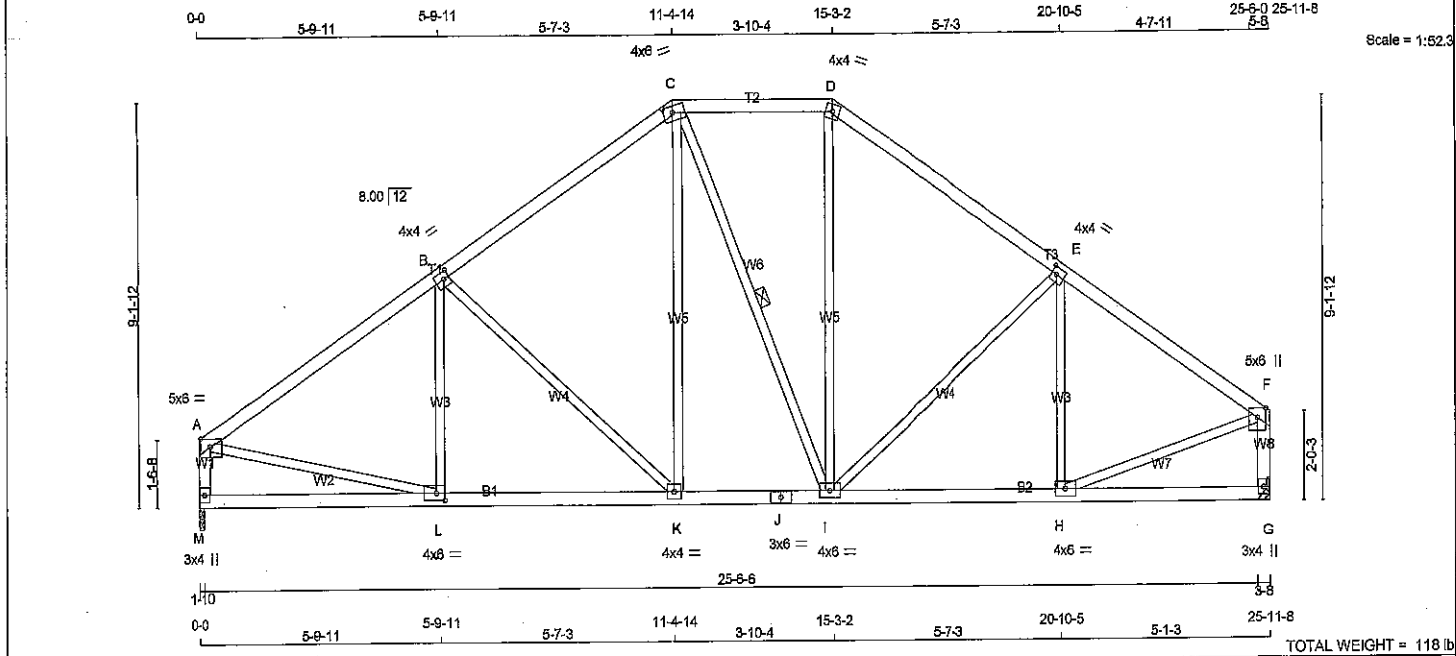
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (M) (INPUT = 0.80)
JSI METAL= 0.47 (K) (INPUT = 1.00)



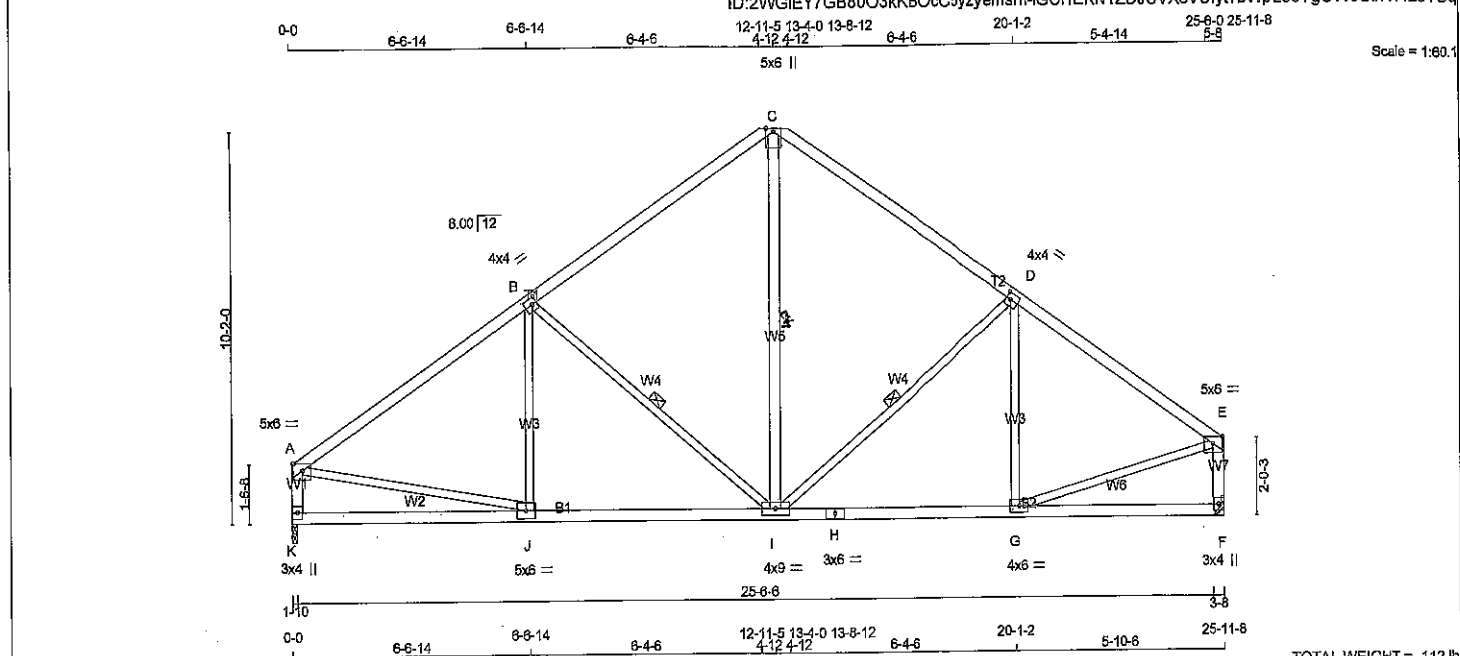
DWG NO. TAM 512B-18
STRUCTURAL
COMPONENT ONLY



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR.				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER										DESIGN CRITERIA				
				BEARINGS										SPECIFIED LOADS:				
A - C	2x4	DRY	No.2	SPF	FACTORED GROSS REACTION					MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG				
C - D	2x4	DRY	No.2	SPF	JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX		IN-SX					
D - F	2x4	DRY	No.2	SPF	M	1457	0	1457	0	0	1-10		1-10					
M - A	2x4	DRY	No.2	SPF	G	1457	0	1457	0	0	HANGER BY OTHERS							
G - F	2x4	DRY	No.2	SPF													MIN. SEAT SIZE: 3-8	
M - J	2x4	DRY	No.2	SPF														
J - G	2x4	DRY	No.2	SPF														
ALL WEBS 2x3 DRY No.2 EXCEPT																SPACING = 24.0 IN. C/C		
DRY: SEASONED LUMBER.																LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12		
																THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC0 2010		
																THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012 , SCBC 2012 , ABC 2014 - CSA 086-09 - TPIC 2011		
																(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD		
																ALLOWABLE DEFL.(LL)= L/360 (0.87") CALCULATED VERT. DEFL.(LL) = L/999 (0.08") ALLOWABLE DEFL.(TL)= L/360 (0.87") CALCULATED VERT. DEFL.(TL) = L/999 (0.11")		
																CSI: TC=0.41/1.00 (A-B:1) , BC=0.40/1.00 (K-L:2) , WB=0.50/1.00 (B-K:1) , SSI=0.20/1.00 (A-B:1)		
																DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10		
																COMPANION LIVE LOAD FACTOR = 0.50		
																TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .		
																NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 618 354 1667 822 2284 1656		
																PLATE PLACEMENT TOL. = 0.250 inches		
																PLATE ROTATION TOL. = 5.0 Deg.		
																JSI GRIP= 0.88 (F) (INPUT = 0.90) JSI METAL = 0.34 (L) (INPUT = 1.00)		
																DWG NO. TAM 5129-18 STRUCTURAL COMPONENT ONLY		



DWG NO. TAM 5129-18
STRUCTURAL
COMPONENT ONLY



LUMBER				N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.	SPF	SPF	SPF	SPF
A - C	2x4	DRY	No.2	SPF	SPF	SPF	SPF
C - E	2x4	DRY	No.2	SPF	SPF	SPF	SPF
K - A	2x4	DRY	No.2	SPF	SPF	SPF	SPF
F - E	2x4	DRY	No.2	SPF	SPF	SPF	SPF
K - H	2x4	DRY	No.2	SPF	SPF	SPF	SPF
H - F	2x4	DRY	No.2	SPF	SPF	SPF	SPF
ALL WEBS 2x3 DRY No.2				SPF	SPF	SPF	SPF
EXCEPT I - C 2x4 DRY No.2				SPF	SPF	SPF	SPF
DRY: SEASONED LUMBER.				SPF	SPF	SPF	SPF

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	6.0	Edge	
B	TMVW-t	MT20	4.0	4.0	2.00	1.50
C	TMTMW+p	MT20	5.0	6.0	Edge	
D	TMVW-t	MT20	4.0	4.0	2.00	1.50
E	TMVW-p	MT20	5.0	6.0	Edge	
F	BMV1+p	MT20	3.0	4.0		
G	BMVW-t	MT20	4.0	6.0		
H	BS-t	MT20	3.0	6.0		
I	BMVWV-t	MT20	4.0	6.0		
J	BMVW-t	MT20	5.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG			
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX		
K	1457	0	1457	0	0	1-10	1-10		
F	1457	0	1457	0	0	HANGER BY OTHERS			
						MIN. SEAT SIZE: 3-8			

UNFACTORED REACTIONS							
JT	1ST LCASE	MAX/MIN	COMPONENT	REACTIONS			
K	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	1196	864 / 0	273 / 0	0 / 0	0 / 0	280 / 0	0 / 0
F	1196	864 / 0	273 / 0	0 / 0	0 / 0	280 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.55 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.
1 LATERAL BRACE(S) AT 1/2 LENGTH OF B-I, D-I, C-I
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	-1619 / 0	-84.3 -84.3	0.56 (1)	J-B	-49 / 239	0.05 (3)	
B-C	-1173 / 0	-84.3 -84.3	0.54 (1)	B-I	-554 / 0	0.30 (1)	
C-D	-1169 / 0	-84.3 -84.3	0.51 (1)	I-D	-426 / 0	0.22 (1)	
D-E	-1450 / 0	-84.3 -84.3	0.49 (1)	G-D	-173 / 162	0.09 (1)	
K-A	-1383 / 0	0.0 0.0	0.14 (1)	A-J	0 / 1403	0.32 (1)	
F-E	-1352 / 0	0.0 0.0	0.16 (1)	G-E	0 / 1325	0.30 (1)	
				I-C	0 / 819	0.13 (1)	
K-J	0 / 0	-28.0 -28.0	0.30 (3)				
J-I	0 / 1379	-28.0 -28.0	0.46 (2)				
I-H	0 / 1273	-28.0 -28.0	0.41 (2)				
H-G	0 / 1273	-28.0 -28.0	0.41 (2)				
G-F	0 / 0	-28.0 -28.0	0.26 (3)				

DESIGN CRITERIA

SPECIFIED LOADS:	
TOP CH. LL	= 25.6 PSF
DL	= 3.0 PSF
BOT CH. LL	= 10.5 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 48.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.87")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.87")
CALCULATED VERT. DEFL.(TL) = L/999 (0.14")

CSI: TC=0.56/1.00 (A-B:1), BC=0.46/1.00 (I-J:2), WB=0.32/1.00 (A-J:1), SS=0.23/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP(DRY)	SHEAR	SECTION		
	(PSI)	(PLI)	(PLI)		
	MAX	MIN	MAX	MIN	MAX
MT20	618	354	1667	822	2284

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (G) (INPUT = 0.90)
JSI METAL= 0.35 (H) (INPUT = 1.00)

DWG NO. TAM 5130-8
STRUCTURAL
COMPONENT ONLY



DWG NO. TAM 5130-18
 STRUCTURAL
 COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
283746	T291	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
Z	23-9-4	—	—	—	BACK	VERT	TOTAL



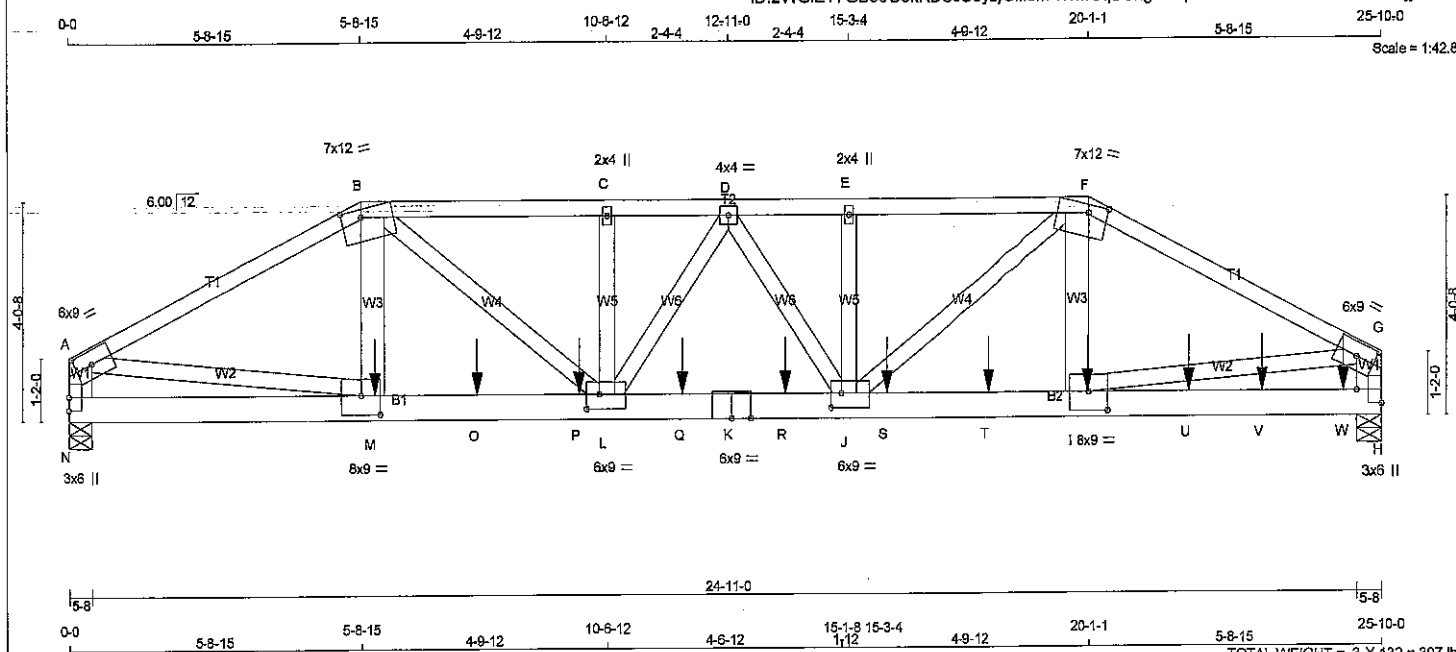
DWG NO. TAM 46136-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
283746	T292	1	3	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF	
B - F	2x4	DRY	No.2	SPF	
F - G	2x4	DRY	No.2	SPF	
N - A	2x6	DRY	No.2	SPF	
H - G	2x6	DRY	No.2	SPF	
N - K	2x6	DRY	2100F 1.8E	SPF	
K - H	2x6	DRY	2100F 1.8E	SPF	

ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
M - B	2x6	DRY	No.2	SPF
I - F	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
A-B	1	12
B-F	1	12
F-G	1	12
N-A	2	12
H-G	2	12
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
N-K	2	9
K-H	2	9
WEBS: (0.122"x3") SPIRAL NAILS		
2x4	1	6
M-B	2	6
2x6	2	6

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMVW-1	MT20	6.0	9.0	3.00 4.00
B	TTWW-m	MT20	7.0	12.0	1.75 4.50
C	TMW-w	MT20	2.0	4.0	
D	TMWW-1	MT20	4.0	4.0	
E	TMW-w	MT20	2.0	4.0	
F	TTWW-m	MT20	7.0	12.0	1.75 4.50
G	TMVW-1	MT20	6.0	9.0	3.00 4.00
H	BMV1+p	MT20	3.0	6.0	3.00 Edge
I	BMWW-1	MT20	6.0	9.0	4.25 4.50
J	BMWWW-1	MT20	6.0	9.0	3.25 2.50
K	BS-1	MT20	6.0	9.0	
L	BMWWW-1	MT20	6.0	9.0	3.25 3.00
M	BMWW-1	MT20	8.0	9.0	4.25 4.50
N	BMV1+p	MT20	3.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
N	8473	0	8473	0	5-8	5-8
H	11409	0	11409	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	8910	3913 / 0	1523 / 0	0 / 0	0 / 0	1475 / 0	0 / 0
H	9289	5284 / 0	2031 / 0	0 / 0	0 / 0	1974 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.44 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CS1 (LC)	UNBRAC LENGTH FR-TO	MEMB.	WEBS MAX. FACTORED FORCE (LBS)	MAX CS1 (LC)
FR-TO							
A-B	-14475 / 0	-84.3	-84.3	0.70 (1)	2.69	M-B	0 / 1902
B-C	-18483 / 0	-84.3	-84.3	0.66 (1)	2.44	B-L	0 / 6511
C-D	-18483 / 0	-84.3	-84.3	0.42 (1)	2.55	L-C	-295 / 27
D-E	-18477 / 0	-84.3	-84.3	0.42 (1)	2.55	J-E	-301 / 24
E-F	-16477 / 0	-84.3	-84.3	0.66 (1)	2.44	J-F	0 / 5458
F-G	-15882 / 0	-84.3	-84.3	0.79 (1)	2.73	I-F	0 / 3222
N-A	-8408 / 0	0.0	0.0	0.18 (1)	6.28	A-M	0 / 13047
H-G	-9032 / 0	0.0	0.0	0.19 (1)	6.07	I-G	0 / 14045
N-M	0 / 0	-28.0	-28.0	0.02 (2)	10.00	L-D	-87 / 0
M-O	0 / 13058	-28.0	-28.0	0.38 (1)	10.00	D-J	-99 / 0
O-P	0 / 13058	-28.0	-28.0	0.38 (1)	10.00		
P-L	0 / 13058	-28.0	-28.0	0.38 (1)	10.00		
L-Q	0 / 18530	-28.0	-28.0	0.44 (1)	10.00		
Q-K	0 / 18530	-28.0	-28.0	0.44 (1)	10.00		
K-R	0 / 18530	-28.0	-28.0	0.44 (1)	10.00		
R-J	0 / 18530	-28.0	-28.0	0.44 (1)	10.00		
J-S	0 / 14130	-28.0	-28.0	0.49 (1)	10.00		
S-T	0 / 14130	-28.0	-28.0	0.49 (1)	10.00		
T-I	0 / 14130	-28.0	-28.0	0.49 (1)	10.00		
I-U	0 / 0	-28.0	-28.0	0.38 (1)	10.00		
U-V	0 / 0	-28.0	-28.0	0.38 (1)	10.00		
V-W	0 / 0	-28.0	-28.0	0.38 (1)	10.00		
W-H	0 / 0	-28.0	-28.0	0.38 (1)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1 MAX-	MAX+	FACE	DIR.	TOTAL
I	20-0-12	-1429	-1429	---	FRONT VERT	TOTAL
M	6-0-4	-2677	-2677	---	FRONT VERT	TOTAL
O	8-0-4	-1441	-1441	---	FRONT VERT	TOTAL
P	10-0-4	-1429	-1429	---	FRONT VERT	TOTAL
Q	12-0-4	-1429	-1429	---	FRONT VERT	TOTAL
R	14-0-4	-1429	-1429	---	FRONT VERT	TOTAL
S	16-0-4	-1429	-1429	---	FRONT VERT	TOTAL
T	18-0-12	-1429	-1429	---	FRONT VERT	TOTAL
U	22-0-12	-1429	-1429	---	FRONT VERT	TOTAL
V	23-6-4	-1429	-1429	---	FRONT VERT	TOTAL
W	25-1-4	-1433	-1433	---	FRONT VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.88")
CALCULATED VERT. DEFL.(LL) = L/999 (0.28")
ALLOWABLE DEFL.(TL) = L/360 (0.88")
CALCULATED VERT. DEFL.(TL) = L/689 (0.45")

CS1: TO=0.79 (F-G:1), BC=0.49 (I-J:1), WB=0.75 (G-I:1), SSI=0.48 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 619 354 1657 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)

JSI METAL= 0.99 (K) (INPUT = 1.00)

DWG NO. TAM46137-17
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
283746	T292	1	3	TRUSS DESC.		

Tamarack Roof Truss, Burlington

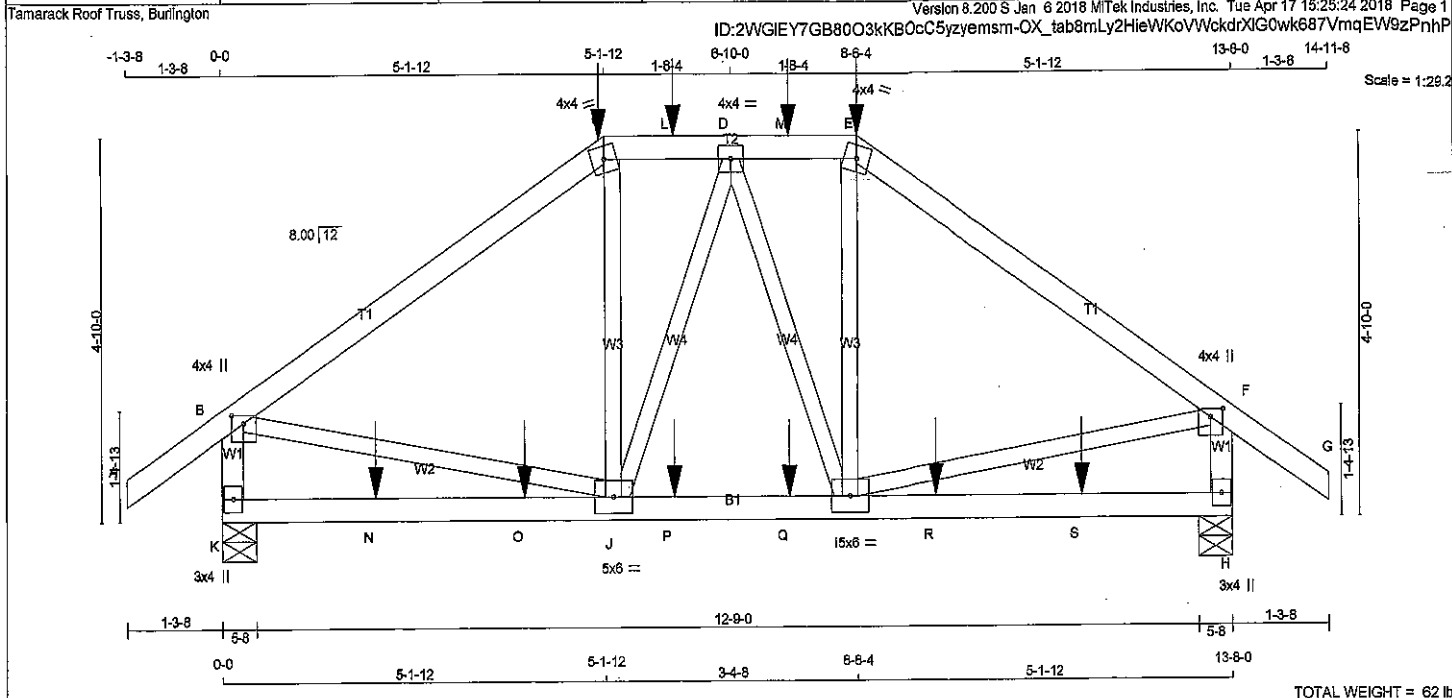
Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Mon Sep 11 14:28:46 2017 Page 2
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Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 2677.3 lbs FACTORED DOWN AT 6-0-4,
1440.8 lbs FACTORED DOWN AT 8-0-4, 1429.0
lbs FACTORED DOWN AT 10-0-4, 1429.0 lbs
FACTORED DOWN AT 12-0-4, 1429.0 lbs
FACTORED DOWN AT 14-0-4, 1429.0 lbs
FACTORED DOWN AT 16-0-4, 1429.0 lbs
FACTORED DOWN AT 18-0-12, 1429.0 lbs
FACTORED DOWN AT 20-0-12, 1429.0 lbs
FACTORED DOWN AT 22-0-12, AND 1429.0 lbs
FACTORED DOWN AT 23-8-4, AND 1432.7 lbs
FACTORED DOWN AT 25-1-4 ON BOTTOM
CHORD. DESIGN FOR UNSPECIFIED
CONNECTION(S) IS DELEGATED TO THE
BUILDING DESIGNER.



DRWG NO. TAM 46137-17
STRUCTURAL
COMPONENT ONLY



LUMBER
N. L. G. A. RULES
CHORDS SIZE LUMBER DESCR.
A - C 2x4 DRY No.2 SPF
C - E 2x4 DRY No.2 SPF
E - G 2x4 DRY No.2 SPF
K - B 2x4 DRY No.2 SPF
H - F 2x4 DRY No.2 SPF
K - H 2x4 DRY No.2 SPF
ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)
JT TYPE PLATES W LEN Y X
B TMVW+p MT20 4.0 4.0 1.25 2.00
C TTW-m MT20 4.0 4.0
D TMWW-t MT20 4.0 4.0
E TTW-m MT20 4.0 4.0
F TMVW+p MT20 4.0 4.0 1.25 2.00
H BMV1+p MT20 3.0 4.0
I BMWW-t MT20 5.0 6.0
J BMWW-t MT20 5.0 8.0
K BMV1+p MT20 3.0 4.0

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 125.3 lbs FACTORED DOWN AT 5-1-12, 125.3 lbs FACTORED DOWN AT 8-6-4, AND 13.1 lbs FACTORED DOWN AT 6-0-12, AND 18.0 lbs FACTORED DOWN AT 7-7-4 ON TOP CHORD, AND 16.4 lbs FACTORED DOWN AT 2-0-12, 16.4 lbs FACTORED DOWN AT 4-0-12, 16.4 lbs FACTORED DOWN AT 6-0-12, 16.4 lbs FACTORED DOWN AT 7-7-4, AND 16.4 lbs FACTORED DOWN AT 9-7-4, AND 16.4 lbs FACTORED DOWN AT 11-7-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
BEARINGS
FACTORED GROSS REACTION
JT VERT HORZ
K 1049 0
H 1049 0
MAXIMUM FACTORED GROSS REACTION
DOWN HORZ UPLIFT IN-SX
1049 0 0 5-8
1049 0 0 5-8
REQRD BRG IN-SX
5-8 5-8

UNFACTORED REACTIONS
1ST LCASE MAX./MIN. COMPONENT REACTIONS
JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL
K 837 504 / 0 165 / 0 0 / 0 0 / 0 169 / 0 0 / 0
H 837 504 / 0 165 / 0 0 / 0 0 / 0 169 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.80 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)
CHORDS
MEMB. FORCE (LBS) FACTORED VERT. LOAD (PLF) MAX. CSI (LC) UNBRAC LENGTH FR-TO
A-B 0 / 32 -84.3 -84.3 0.12 (1) 10.00 J-C 0 / 191 0.05 (3)
B-C -892 / 0 -84.3 -84.3 0.46 (1) 5.80 J-D -89 / 0 0.04 (1)
C-L -744 / 0 -84.3 -84.3 0.04 (1) 6.25 D-I -90 / 0 0.04 (1)
L-D -744 / 0 -84.3 -84.3 0.04 (1) 6.25 I-E 0 / 191 0.05 (3)
D-M -744 / 0 -84.3 -84.3 0.04 (1) 6.25 B-J 0 / 758 0.19 (1)
M-E -744 / 0 -84.3 -84.3 0.04 (1) 6.25 I-F 0 / 758 0.19 (1)
E-F -891 / 0 -84.3 -84.3 0.46 (1) 5.80
F-G 0 / 32 -84.3 -84.3 0.12 (1) 10.00
K-B -979 / 0 0.0 0.0 0.11 (1) 7.81
H-F -978 / 0 0.0 0.0 0.11 (1) 7.81
K-N 0 / 0 -28.0 -28.0 0.22 (2) 10.00
N-O 0 / 0 -28.0 -28.0 0.22 (2) 10.00
O-Q 0 / 0 -28.0 -28.0 0.22 (2) 10.00
J-P 0 / 772 -28.0 -28.0 0.28 (2) 10.00
P-Q 0 / 772 -28.0 -28.0 0.28 (2) 10.00
Q-I 0 / 772 -28.0 -28.0 0.28 (2) 10.00
I-R 0 / 0 -28.0 -28.0 0.22 (2) 10.00
R-S 0 / 0 -28.0 -28.0 0.22 (2) 10.00
S-H 0 / 0 -28.0 -28.0 0.22 (2) 10.00

FACTORED CONCENTRATED LOADS (LBS)
JT LOC. LC1 MAX- MAX+ FACE DIR.
C 5-1-12 -11 -13 FRONT VERT
C 5-1-12 -114 -114 FRONT VERT
E 8-6-4 -11 -13 FRONT VERT
E 8-6-4 -114 -114 FRONT VERT
L 8-0-12 -13 -13 FRONT VERT
M 7-7-4 -13 -13 FRONT VERT
N 2-0-12 -9 -16 FRONT VERT
O 4-0-12 -9 -16 FRONT VERT
P 6-0-12 -9 -16 FRONT VERT
Q 7-7-4 -9 -16 FRONT VERT
R 9-7-4 -9 -16 FRONT VERT
S 11-7-4 -9 -16 FRONT VERT

DESIGN CRITERIA
*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.46")
CALCULATED VERT. DEFL.(LL) = L/989 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.46")
CALCULATED VERT. DEFL.(TL) = L/999 (0.07")

CSI: TC=0.48/1.00 (B-C:1), BC=0.28/1.00 (I-J:2), WB=0.19/1.00 (B-J:1), SSI=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

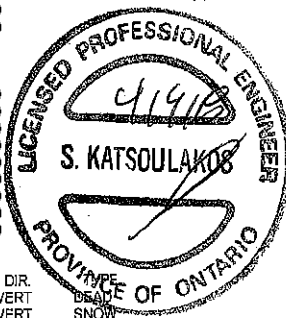
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1967 622 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.88 (F) (INPUT = 0.90)
JSI METAL= 0.25 (F) (INPUT = 1.00)

DWG NO. TAM 21167-10
STRUCTURAL
COMPONENT ONLY



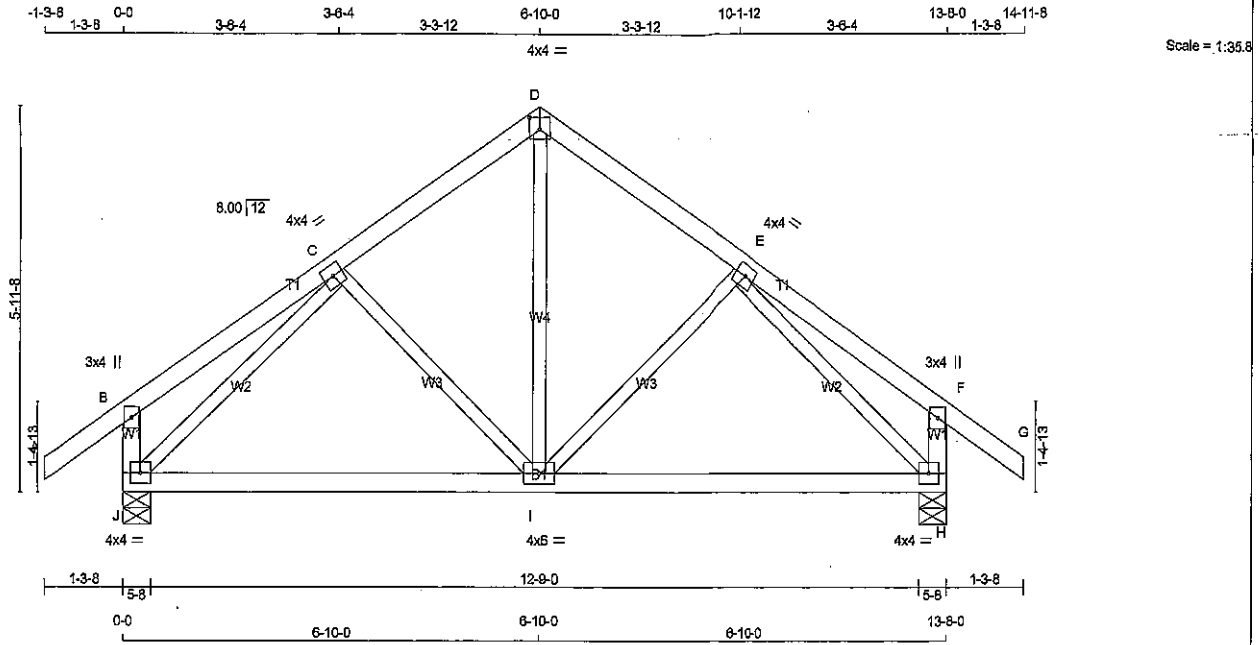
DRWG NO. TAM 21161-12
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286076	T294	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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Scale = 1:35.8

TOTAL WEIGHT = 2 X 59 = 119 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	8.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS							
FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	883	0	883	0	0	5-8	5-8
H	883	0	883	0	0	5-8	5-8

UNFACTORED REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	708	420 / 0	144 / 0	0 / 0	0 / 0	145 / 0	0 / 0
H	708	420 / 0	144 / 0	0 / 0	0 / 0	145 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING							
TOTAL LOAD CASES: (4)							
CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)	
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	C-I	-140 / 44
B-C	0 / 18	-84.3	-84.3	0.15 (1)	10.00	I-D	0 / 432
C-D	-510 / 0	-84.3	-84.3	0.12 (1)	6.25	I-E	-139 / 44
D-E	-810 / 0	-84.3	-84.3	0.12 (1)	5.25	J-C	-831 / 0
E-F	0 / 18	-84.3	-84.3	0.15 (1)	10.00	E-H	-831 / 0
F-G	0 / 32	-84.3	-84.3	0.11 (1)	10.00		
J-B	-227 / 0	0.0	0.0	0.02 (1)	7.81		
H-F	-227 / 0	0.0	0.0	0.02 (1)	7.81		
J-I	0 / 592	-28.0	-28.0	0.44 (2)	10.00		
I-H	0 / 592	-28.0	-28.0	0.44 (2)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TRC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.46")
CALCULATED VERT. DEFL.(LL) = L/989 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (0.46")
CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CSI: TC=0.15/1.00 (B-C:1), BC=0.44/1.00 (I-J:2), WB=0.30/1.00 (C-J:1), SS=0.18/1.00 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
OOMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP(DRY)	SHEAR	SECTION	(PSI)	(PL)
MT20	518	354	1657	622	2284

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.73 (D) (INPUT = 0.90)
JSI METAL= 0.28 (C) (INPUT = 1.00)



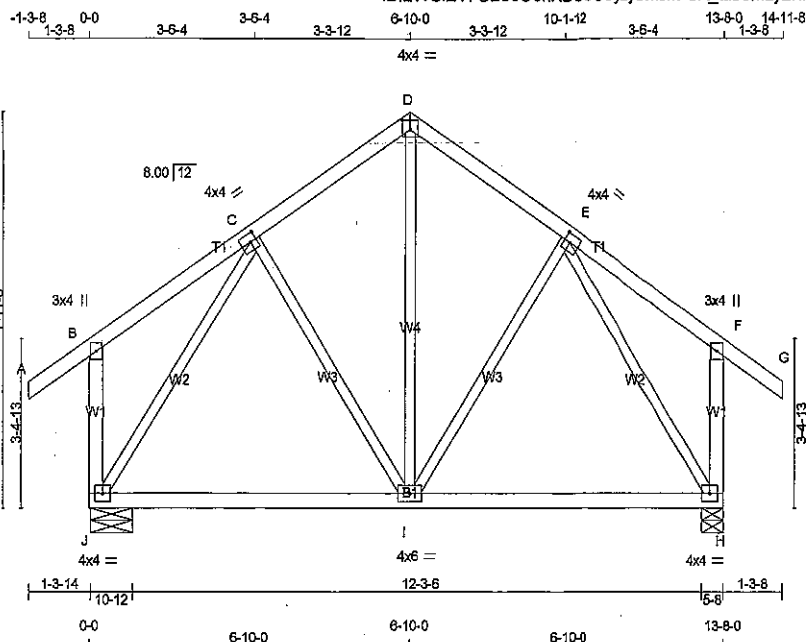
DRWG NO. TAM 21162-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286076	T295	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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Scale = 1:48.5

LUMBER					LUMBER		DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	DRY	No.2	SPF			
A - D	2x4	DRY	No.2	SPF				
D - G	2x4	DRY	No.2	SPF				
J - B	2x4	DRY	No.2	SPF				
H - F	2x4	DRY	No.2	SPF				
J - H	2x4	DRY	No.2	SPF				
ALL WEBS	2x3	DRY	No.2	SPF				
EXCEPT								

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0	2.00	1.50
F	TMV+p	MT20	3.0	4.0		
H	BMWW-t	MT20	4.0	4.0		
I	BMWW-t	MT20	4.0	6.0		
J	BMWW-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
J	883	0	883	0
H	883	0	883	0

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	708	420 / 0	144 / 0	0 / 0	0 / 0	145 / 0	0 / 0
H	708	420 / 0	144 / 0	0 / 0	0 / 0	145 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED	FACTORED		MEMB.	MAX. FACTORED		
	FORCE	VERT. LOAD			FORCE		
	(LBS)	(PLF)	CSI (LC)		(LBS)	CSI (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 32	-84.3 -84.3	0.11 (1)	10.00	C-I	-14 / 95	0.02 (3)
B-C	0 / 19	-84.3 -84.3	0.16 (1)	10.00	I-D	0 / 294	0.07 (2)
C-D	-456 / 0	-84.3 -84.3	0.12 (1)	6.25	I-E	-14 / 95	0.02 (3)
D-E	-456 / 0	-84.3 -84.3	0.12 (1)	6.25	J-C	-893 / 0	0.48 (1)
E-F	0 / 19	-84.3 -84.3	0.16 (1)	10.00	E-H	-893 / 0	0.48 (1)
F-G	0 / 32	-84.3 -84.3	0.11 (1)	10.00			
J-B	-225 / 0	0.0 0.0	0.04 (1)	7.81			
H-F	-225 / 0	0.0 0.0	0.04 (1)	7.81			
J-I	0 / 372	-28.0 -28.0	0.42 (2)	10.00			
I-H	0 / 371	-28.0 -28.0	0.42 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.46")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.46")
CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CSI: TC=0.16/1.00 (B-C:1), BC=0.42/1.00 (I-J:2), WB=0.48/1.00 (C-J:1), SSI=0.16/1.00 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	618	354	1667

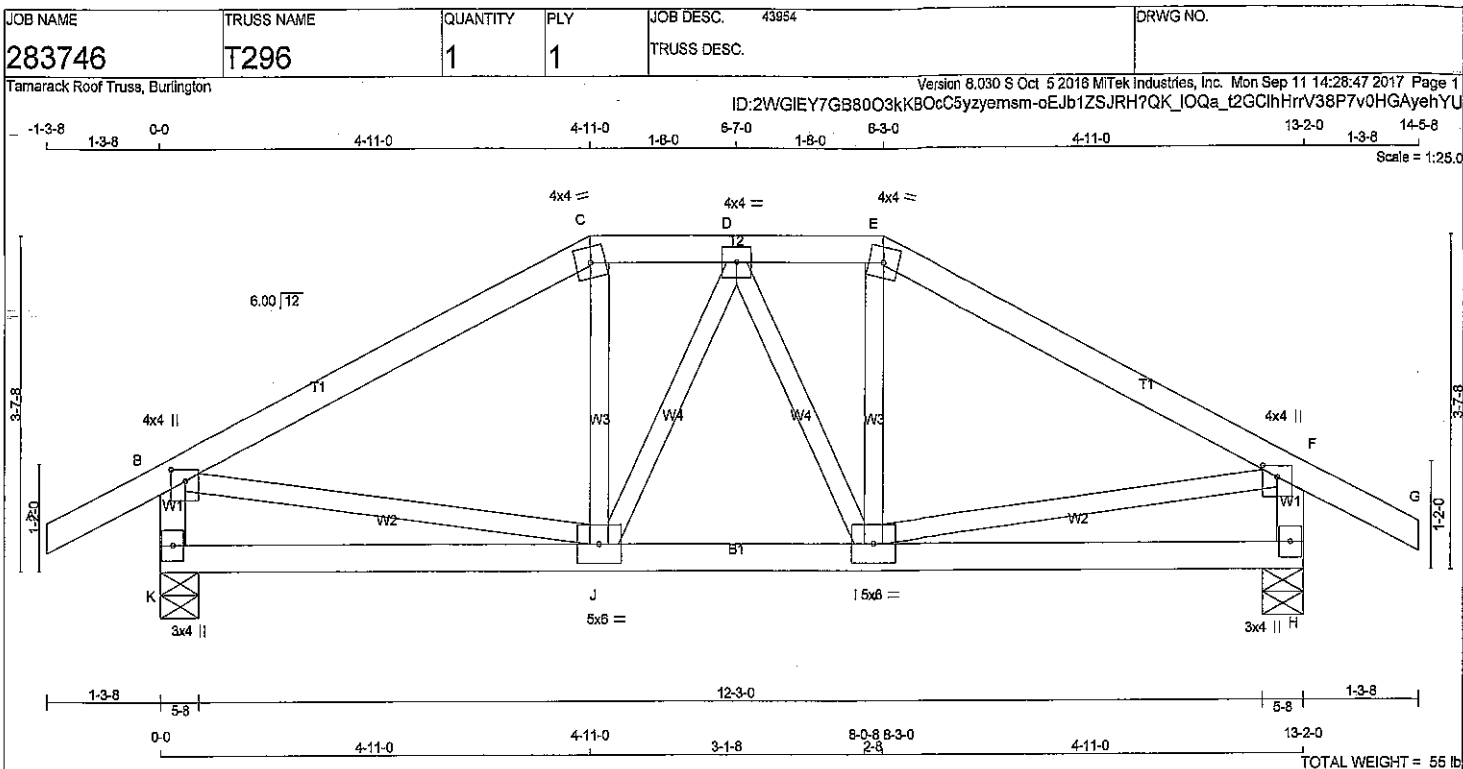
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (E) (INPUT = 0.90)
JSI METAL= 0.24 (C) (INPUT = 1.00)



DRWG NO. TAM216318
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - E	2x4	DRY No.2	SPF
E - G	2x4	DRY No.2	SPF
K - B	2x4	DRY No.2	SPF
H - F	2x4	DRY No.2	SPF
K - H	2x4	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TM/VW+p	MT20	4.0	4.0	1.50	2.00
C	TTW-m	MT20	4.0	4.0		
D	TM/VW-t	MT20	4.0	4.0		
E	TTW-m	MT20	4.0	4.0		
F	TM/VW+p	MT20	4.0	4.0	1.50	2.00
H	BM/V1+p	MT20	3.0	4.0		
I	GM/VWV-t	MT20	5.0	8.0		
J	BM/VWV-t	MT20	5.0	8.0		
K	BM/V1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
K	853	0	853	0
H	853	0	853	0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	684	406 / 0	138 / 0	0 / 0	0 / 0	140 / 0	0 / 0
H	684	408 / 0	138 / 0	0 / 0	0 / 0	140 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 26	-84.3 -84.3	0.11 (1)	10.00	J-C	0 / 156	0.04 (3)
B-C	-785 / 0	-84.3 -84.3	0.27 (1)	6.25	I-E	0 / 156	0.04 (3)
C-D	-705 / 0	-84.3 -84.3	0.03 (1)	6.25	B-J	0 / 711	0.16 (1)
D-E	-705 / 0	-84.3 -84.3	0.03 (1)	6.25	I-F	0 / 711	0.16 (1)
E-F	-785 / 0	-84.3 -84.3	0.27 (1)	8.25	J-D	-85 / 0	0.02 (1)
F-G	0 / 26	-84.3 -84.3	0.11 (1)	10.00	D-I	-85 / 0	0.02 (1)
K-B	-792 / 0	0.0 0.0	0.08 (1)	7.81			
H-F	-792 / 0	0.0 0.0	0.08 (1)	7.81			
K-J	0 / 0	-28.0 -28.0	0.16 (2)	10.00			
J-I	0 / 740	-28.0 -28.0	0.22 (2)	10.00			
I-H	0 / 0	-28.0 -28.0	0.16 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.8 PSF
	DL = 3.0 PSF
BOT CH.	LL = 10.5 PSF
	DL = 7.0 PSF
TOTAL LOAD	= 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014

- CSA C88-09

- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.44")

CALCULATED VERT. DEFL.(LL) = L/999 (0.03")

ALLOWABLE DEFL.(TL) = L/360 (0.44")

CALCULATED VERT. DEFL.(TL) = L/999 (0.06")

CSI: TC=0.27 (B-C:1), BC=0.22 (I-J:2), WB=0.16 (B-J:1), SSI=0.14 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
MT20	818 354	1687 622 2284 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)

JSI METAL= 0.23 (B) (INPUT = 1.00)

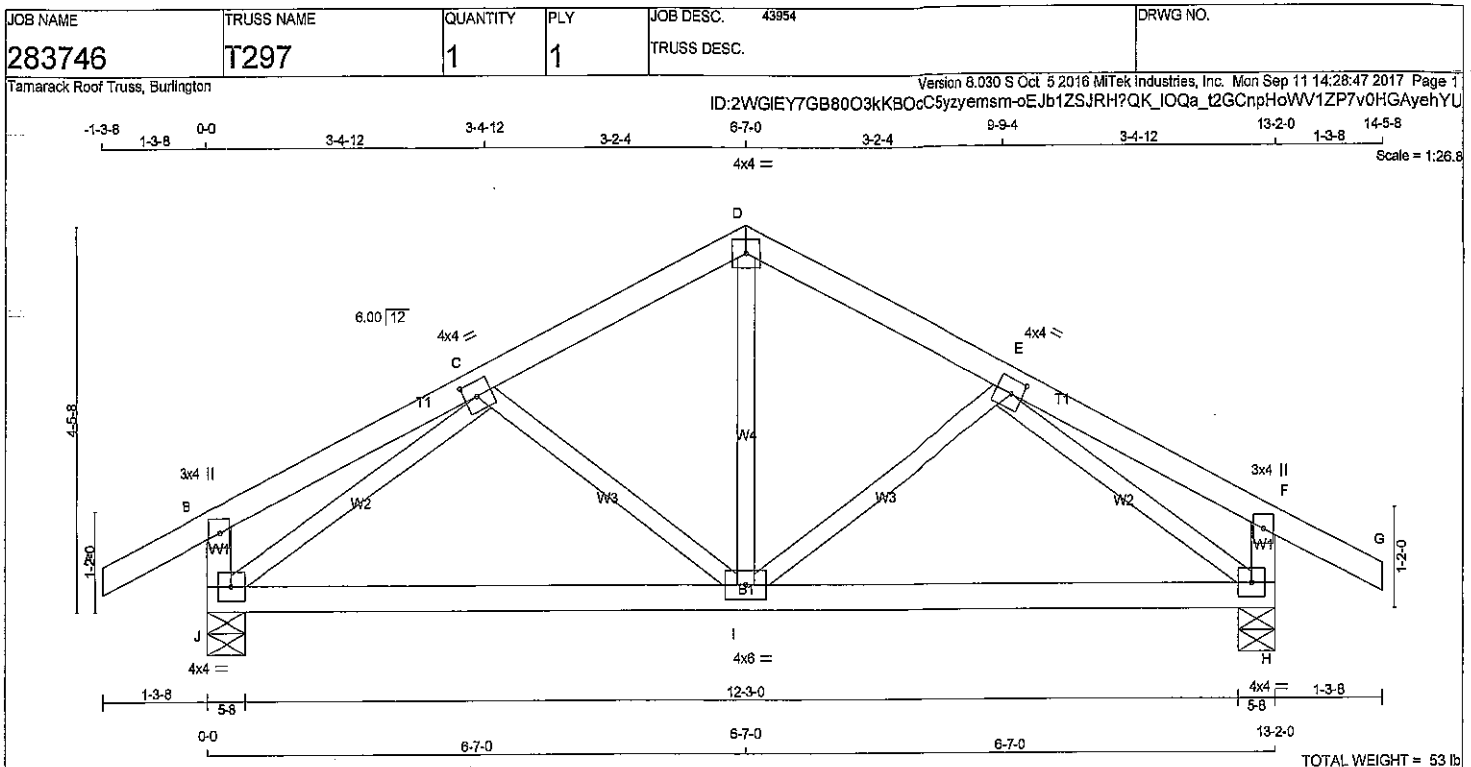
DWG NO. TAM 4641-17

STRUCTURAL

COMPONENT ONLY



DWG NO. TAM46141-17
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTW-p	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.75
F	TMV+p	MT20	3.0	4.0		
H	BMVW-t	MT20	4.0	4.0		
I	BMWW-t	MT20	4.0	6.0		
J	BMVW-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
J	853	0	853	0	5-8	5-8
H	853	0	853	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LOASE MAX./MIN. COMPONENT REACTIONS						
	COMBINED	SNOW	LIVE	PERMLIVE	WIND	DEAD	SOIL
J	884	408 / 0	138 / 0	0 / 0	0 / 0	140 / 0	0 / 0
H	884	408 / 0	138 / 0	0 / 0	0 / 0	140 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1		MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
		FROM	TO				
FR-TO					FR-TO		
A-B	0 / 26	-84.3	-84.3	0.11 (1)	10.00	I-D	0 / 403
B-C	0 / 13	-84.3	-84.3	0.13 (1)	10.00	I-E	-145 / 55
C-D	-707 / 0	-84.3	-84.3	0.11 (1)	6.25	C-I	-145 / 55
D-E	-707 / 0	-84.3	-84.3	0.11 (1)	6.25	J-C	-923 / 0
E-F	0 / 13	-84.3	-84.3	0.13 (1)	10.00	E-H	-923 / 0
F-G	0 / 26	-84.3	-84.3	0.11 (1)	10.00		
J-B	-224 / 0	0.0	0.0	0.02 (1)	7.81		
H-F	-224 / 0	0.0	0.0	0.02 (1)	7.81		
J-I	0 / 735	-28.0	-28.0	0.43 (2)	10.00		
I-H	0 / 735	-28.0	-28.0	0.43 (2)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 3.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.44")

CALCULATED VERT. DEFL.(LL) = L/999 (0.08")

ALLOWABLE DEFL.(TL) = L/360 (0.44")

CALCULATED VERT. DEFL.(TL) = L/999 (0.10")

CSI: TC=0.13 (B-C:1), BC=0.43 (I-J:2), WB=0.26 (C-J:1), SSI=0.16 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1687
	822	2284	1658

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.82 (H) (INPUT = 0.90)

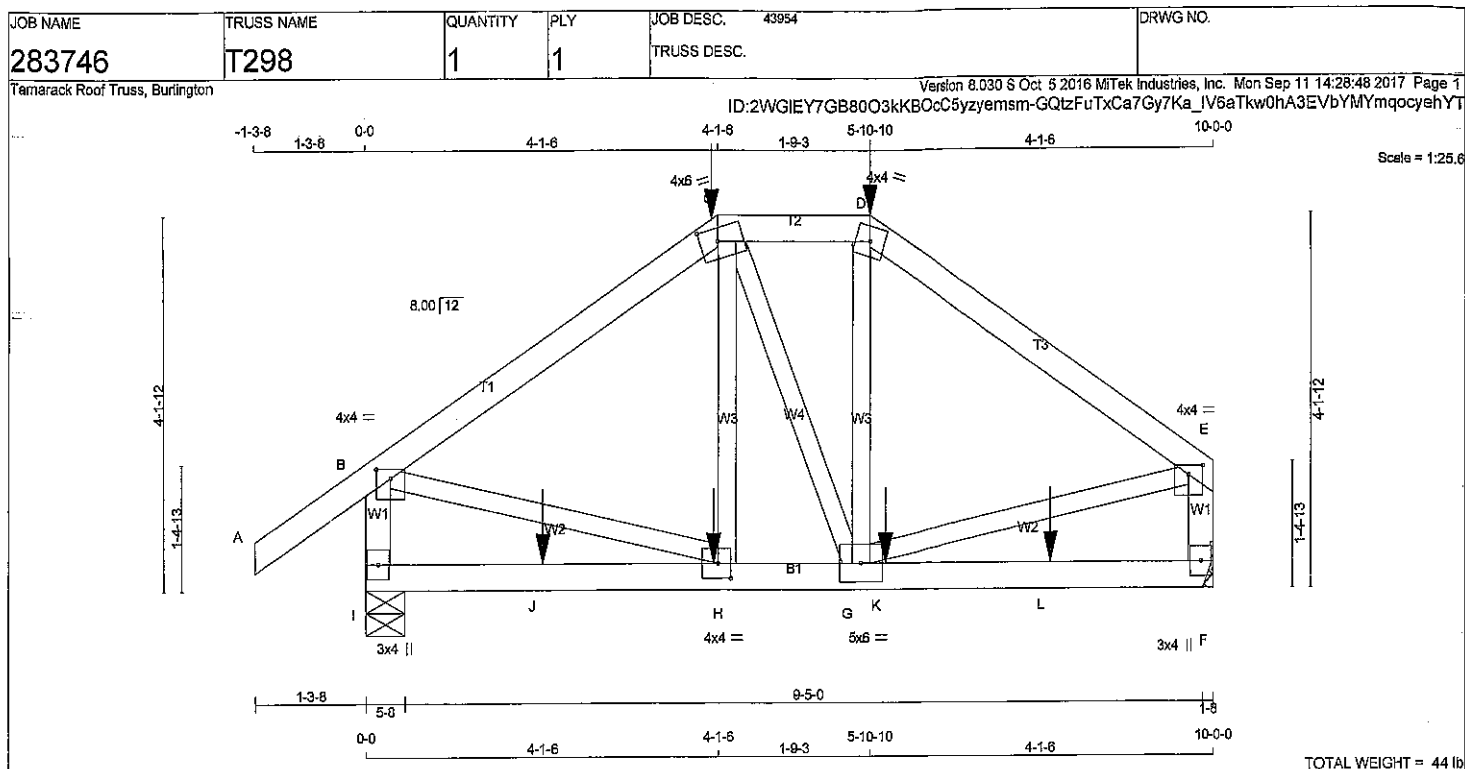
JSI METAL= 0.32 (E) (INPUT = 1.00)



DWG NO. TAM 46143-17

STRUCTURAL

COMPONENT ONLY



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
I - B	2x4	DRY	No.2	SPF	
F - E	2x4	DRY	No.2	SPF	
I - F	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	4.0	1.25	2.00
C	TTWW-m	MT20	4.0	6.0	1.75	2.50
D	TTWW-m	MT20	4.0	4.0		
E	TMVW-p	MT20	4.0	4.0	1.25	2.00
F	BMVW-p	MT20	3.0	4.0		
G	BMVW-w	MT20	5.0	6.0		
H	BMVW-w	MT20	4.0	4.0	2.00	1.75
I	BMVW-p	MT20	3.0	4.0		

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 256.3 lbs FACTORED DOWN AT 4-1-6, 256.3 lbs FACTORED DOWN AT 5-10-10, AND 107.6 lbs FACTORED DOWN AT 4-1-8, AND 107.6 lbs FACTORED DOWN AT 5-10-10 ON TOP CHORD, AND 71.5 lbs FACTORED DOWN AT 2-0-12, 71.5 lbs FACTORED DOWN AT 4-0-12, AND 71.5 lbs FACTORED DOWN AT 6-0-12, AND 71.5 lbs FACTORED DOWN AT 8-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	UPLIFT	IN-SX
I	1121	0	0	5-8
F	1008	0	0	5-8

HANGER BY OTHERS
MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
I	887	544 / 0	165 / 0	0 / 0	0 / 0	177 / 0	0 / 0	0 / 0
F	812	474 / 0	168 / 0	0 / 0	0 / 0	171 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.84 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

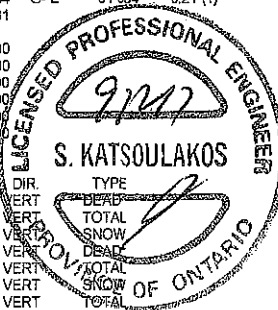
MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)
FR-TO									
A-B	0 / 32	-84.3	-84.3	0.12 (1)	10.00	H-C	-54 / 164	0.04 (3)	
B-C	-993 / 0	-84.3	-84.3	0.30 (1)	5.85	C-G	0 / 16	0.00 (3)	
C-D	-825 / 0	-84.3	-84.3	0.08 (1)	6.25	G-D	-57 / 181	0.04 (3)	
D-E	-996 / 0	-84.3	-84.3	0.30 (1)	5.84	B-H	0 / 851	0.21 (1)	
I-B	-1053 / 0	0.0	0.0	0.12 (1)	7.64	G-E	0 / 854	0.21 (1)	
F-E	-937 / 0	0.0	0.0	0.11 (1)	7.81				
J-H	0 / 0	-28.0	-28.0	0.20 (2)	10.00				
H-G	0 / 823	-28.0	-28.0	0.28 (2)	10.00				
G-K	0 / 0	-28.0	-28.0	0.21 (2)	10.00				
K-L	0 / 0	-28.0	-28.0	0.21 (2)	10.00				
L-F	0 / 0	-28.0	-28.0	0.21 (2)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE
C	4-1-6	-24	-27	---	FRONT	VERT	DEAD
C	4-1-6	-108	-108	---	BACK	VERT	TOTAL
C	4-1-6	-233	-233	---	FRONT	VERT	SNOW
D	5-10-10	-24	-27	---	FRONT	VERT	DEAD
D	5-10-10	-107	-107	---	BACK	VERT	TOTAL
D	5-10-10	-233	-233	---	FRONT	VERT	SNOW
H	4-0-12	-41	-71	---	BACK	VERT	TOTAL
J	2-0-12	-41	-71	---	BACK	VERT	TOTAL
K	6-0-12	-41	-71	---	BACK	VERT	TOTAL
L	8-0-12	-41	-71	---	BACK	VERT	TOTAL

WEBS

MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (PLF)
H-C	-54 / 164	0.04 (3)
C-G	0 / 16	0.00 (3)
G-D	-57 / 181	0.04 (3)
B-H	0 / 851	0.21 (1)
G-E	0 / 854	0.21 (1)



DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
	DL = 3.0 PSF
BOT CH.	LL = 10.5 PSF
	DL = 7.0 PSF
TOTAL LOAD	= 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***

ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(5% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.33")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.33")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.30 (D-E:1), BC=0.28 (G-H:2), WB=0.21 (E-G:1), SSI=0.20 (F-G:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(FSL)	(PL)	(PL)	(PL)
MAX	MIN	MAX	MIN
MT20	618	354	1697
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (B) (INPUT = 0.90)
JSI METAL= 0.31 (H) (INPUT = 1.00)

DWG NO. TAM46144-17
STRUCTURAL
COMPONENT ONLY

LUL/LUS/LJS/HUS/HHUS/HGUS

SIMPSON
Strong-Tie

Standard and Double-Shear Joist Hangers



This product is preferable to similar connectors because of
a) easier installation, b) higher capacities, c) lower installed
cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 258–259.

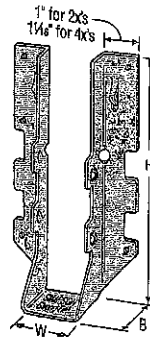
Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 20–24.

Installation:

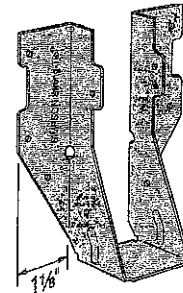
- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1 1/2" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

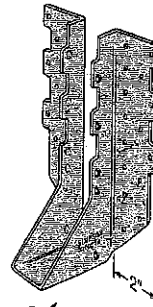
- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on p. 126.



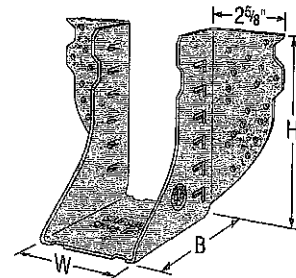
✓ LUS28



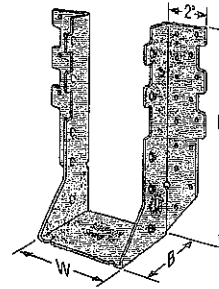
LU26L



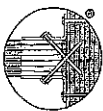
✓ HUS210
(HUS26, HUS28,
and HHUS similar)



✓ HGUS28-2



✓ HHUS210-2



Double-Shear
Nailing
Top View

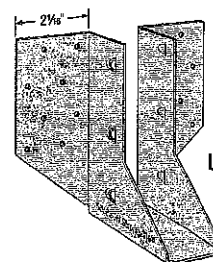
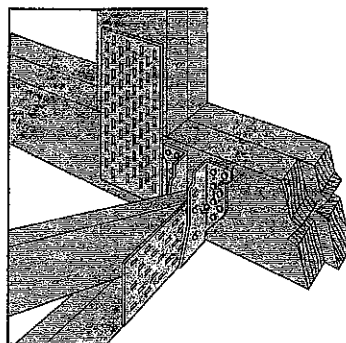


Double-Shear
Nailing
Side View;
Do not
bend tab



Dome Double-Shear
Nailing Side View
(available on
some models)
U.S. Patent 5,603,580

**Typical HUS26
Installation
with Reduced
Heel Height**
(Truss Designer
to provide
fastener quantity
for connecting
multiple members
together)



LJS26DS

Plated Truss Connectors

LUL/LUS/LJS/HUS/HHUS/HGUS

HHUS/HGUS

See Hanger Options information on pp. 125–127.

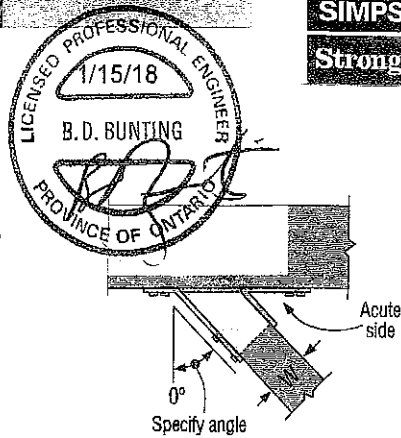
HHUS — Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.62 of the table value
- The joist must be bevel-cut to allow for double-shear nailing

HGUS — Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Joist	Down Resistance	Uplift
W < 2"	Bevel or square cut	0.62 of table value	0.46 of table value
2" < W < 6"	Bevel cut	0.67 of table value	0.41 of table value
2" < W < 6"	Square cut	0.46 of table value	0.41 of table value
W > 6"	Bevel cut	0.75 of table value	0.41 of table value



Top View HHUS Hanger
Skewed Right
(Joist must be bevel cut)
All joist nails installed on the
outside angle (non-acute side).

Standard and Double-Shear Joist Hangers (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (In.)				Fasteners		Factored Resistance					
		W	H	B	d _g ³	Header	Joist	D-Fir-L		S-P-F			
								Uplift (K _D = 1.15)	Normal (K _D = 1.00)	Uplift (K _D = 1.15)	Normal (K _D = 1.00)		
								lb.	lb.	lb.	lb.		
										lb.	lb.	lb.	lb.
										kN	kN	kN	kN
Single 2x Sizes													
LUS24	18	1 1/8	3 1/8	1 3/4	2 1/4	(4) 10d	(2) 10d	710	1625	645	1155		
								3.16	7.23	2.87	5.14		
LU24L	22	1 1/8	3	1 1/8	2 1/8	(4) 10d	(2) 10d x 1 1/2"	380	1020	320	725		
								1.60	4.54	1.42	3.22		
LU26L	22	1 1/8	5	1 1/8	4 1/8	(6) 10d	(4) 10d x 1 1/2"	720	1605	645	1140		
								3.20	7.14	2.87	5.07		
SS LUS26	18	1 1/8	4 3/4	1 3/4	3 3/4	(4) 10d	(4) 10d	1420	2170	1290	1630		
								6.32	9.85	5.74	7.25		
HUS26	16	1 1/8	5 3/8	3	3 1/8	(14) 16d	(6) 16d	2705	4940	2065	3875		
								11.30	21.97	9.20	17.24		
LJS26DS	18	1 1/8	5	3 1/2	4 1/8	(16) 16d	(6) 16d	2055	4265	1460	4115		
								9.14	18.97	6.49	18.31		
HGUS26	12	1 1/8	5 3/8	5	4 1/8	(20) 16d	(8) 16d	2685	6625	2685	5700		
								11.96	29.51	11.96	25.35		
LU28L	20	1 1/8	6 3/4	1 1/8	5 1/8	(8) 10d	(5) 10d x 1 1/2"	1140	2185	1020	1550		
								5.07	9.72	4.54	6.89		
SS LUS28	18	1 1/8	6 3/4	1 3/4	3 3/4	(6) 10d	(4) 10d	1420	2520	1290	1790		
								6.32	11.21	5.74	7.96		
HUS28	16	1 1/8	7 1/8	3	6 1/8	(22) 16d	(8) 16d	3605	5365	2675	4345		
								16.04	23.86	11.90	19.33		
HGUS28	12	1 1/8	7 1/8	5	6 1/8	(36) 16d	(12) 16d	3310	7675	3310	6900		
								14.74	34.19	14.74	30.73		
LU210L	20	1 1/8	8	1 1/8	7 1/8	(10) 10d	(6) 10d x 1 1/2"	1140	2495	1020	1770		
								5.07	11.10	4.54	7.87		
SS LUS210	18	1 1/8	7 1/8	1 3/4	3 3/4	(8) 10d	(4) 10d	1420	2785	1290	2210		
								6.32	12.39	5.74	9.83		

1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.
3. d_g is the distance from the bearing seat to the top joist nail.
4. Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-C-N10TRSSCN and/or see installation notes.
5. Nails: 16d = 0.162" dia. x 3 1/2" long. See pp. 27–28 for other nail sizes and information.

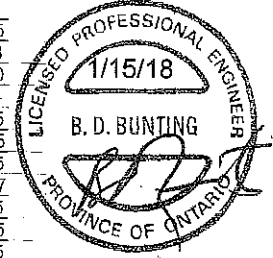
Face-Mount Hangers

SIMPSON
Strong-Tie

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance			
		W	H	B	d _g ³	Header	Joist	D-Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
								(K _D = 1.15)	(K _D = 1.00)	(K _D = 1.15)	(K _D = 1.00)
								lb.	lb.	lb.	lb.
								kN	kN	kN	kN
Double 2x Sizes											
LUS24-2	18	3 3/8	3 3/8	2	1 1/4	(4) 16d	(2) 16d	835	2020	590	1435
								3.71	8.99	2.62	6.38
LUS26-2	18	3 3/8	4 3/8	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
								7.65	11.54	6.87	8.54
HHUS26-2	14	3 3/8	5 3/8	3	3 3/8	(14) 16d	(6) 16d	2850	7335	2065	5205
								12.68	32.63	9.20	23.15
HGUS26-2	12	3 3/8	5 3/8	4	4 3/8	(20) 16d	(8) 16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
LUS28-2	18	3 3/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
								7.65	14.79	6.87	11.45
HHUS28-2	14	3 3/8	7 3/8	3	6 3/8	(22) 16d	(8) 16d	3765	8940	2675	6345
								16.75	39.77	11.90	28.22
HGUS28-2	12	3 3/8	7 3/8	4	6 3/8	(36) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
LUS210-2	18	3 3/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
								11.48	20.02	10.32	14.21
HHUS210-2	14	3 3/8	9 3/8	3	8	(30) 16d	(10) 16d	4670	9660	4235	7000
								20.77	42.97	18.84	31.14
HGUS210-2	12	3 3/8	9 3/8	4	8 3/8	(46) 16d	(16) 16d	6840	14015	4855	10270
								30.43	62.34	21.60	45.69
Triple 2x Sizes											
HGUS26-3	12	4 1/8	5 1/2	4	4 3/8	(20) 16d	(8) 16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
HGUS28-3	12	4 1/8	7 1/4	4	6 3/8	(36) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
HHUS210-3	14	4 1/8	9	3	7 1/8	(30) 16d	(10) 16d	4670	9670	4235	8665
								20.77	43.02	18.84	30.54
HGUS210-3	12	4 1/8	9 1/4	4	8 3/8	(46) 16d	(16) 16d	6840	14645	4855	10400
								30.43	65.14	21.60	46.26
Quadruple 2x Sizes											
HGUS26-4	12	6 3/8	5 3/8	4	4 3/8	(20) 16d	(8) 16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
HGUS28-4	12	6 3/8	7 3/8	4	6 3/8	(36) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
HHUS210-4	14	6 3/8	8 3/8	3	7 1/8	(30) 16d	(10) 16d	4670	10155	4235	7210
								20.77	45.17	18.84	32.07
HGUS210-4	12	6 3/8	9 3/8	4	8 3/8	(46) 16d	(16) 16d	6840	14645	4855	10400
								30.43	65.14	21.60	46.26
HGUS212-4	12	6 3/8	10 3/8	4	10 3/8	(56) 16d	(20) 16d	7640	14995	5425	10645
								33.98	66.70	24.13	47.35
HGUS214-4	12	6 3/8	12 3/8	4	11 3/8	(66) 16d	(22) 16d	10130	16400	7195	11645
								45.06	72.95	32.00	51.80
4x Sizes											
LUS46	18	3 3/8	4 3/8	2	3 3/8	(4) 16d	(4) 16d	1720	2595	1545	1920
								7.65	11.54	6.87	8.54
HHUS46	14	3 3/8	5 3/8	3	3 3/8	(14) 16d	(6) 16d	2540	7335	2065	5205
								11.30	32.63	9.20	23.15
HGUS46	12	3 3/8	5 3/8	4	4 3/8	(20) 16d	(8) 16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
LUS48	18	3 3/8	6 3/8	2	3 3/8	(6) 16d	(4) 16d	1720	3325	1545	2575
								7.65	14.79	6.87	11.45
HHUS48	14	3 3/8	7 3/8	3	6 3/8	(22) 16d	(8) 16d	3765	8940	2675	6345
								16.75	39.77	11.90	28.22
HGUS48	12	3 3/8	7 3/8	4	6 3/8	(36) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
LUS410	18	3 3/8	8 3/8	2	5 3/8	(8) 16d	(6) 16d	2580	4500	2320	3195
								11.48	20.02	10.32	14.21
HGUS410	12	3 3/8	9	4	8 3/8	(46) 16d	(16) 16d	6840	14015	4855	10270
								30.43	62.34	21.60	45.69
HGUS412	12	3 3/8	10 3/8	4	10 3/8	(56) 16d	(20) 16d	7640	14995	5425	10645
								33.98	66.70	24.13	47.35
HGUS414	12	3 3/8	12 3/8	4	11 3/8	(66) 16d	(22) 16d	10130	16400	7195	11645
								45.06	72.95	32.00	51.80

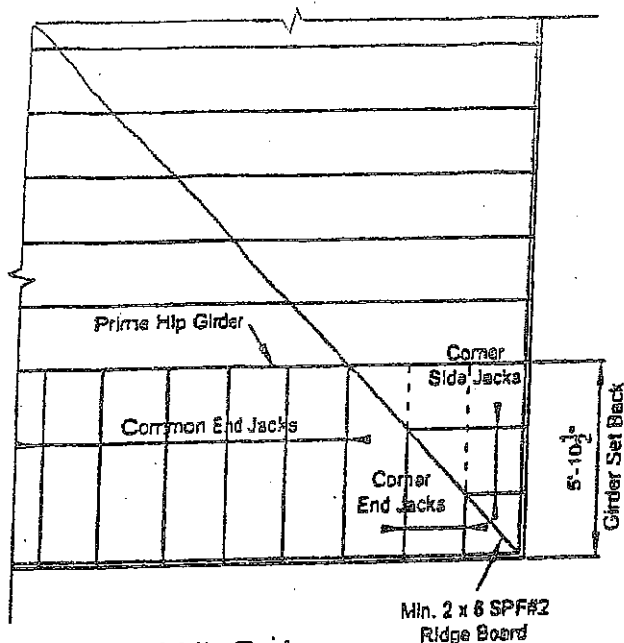


Plated Truss Connectors

See footnotes
on p. 258.

MICRO CITY ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242



45° Hip End

LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2

BOTTOM CHORD : 2 x 4 SPF#2

WEBS : 2 x 3 SPF#2

UNLESS OTHERWISE SHOWN

DESIGN LOAD:

TOP CHORD LIVE LOAD : 34.8 P.S.F.

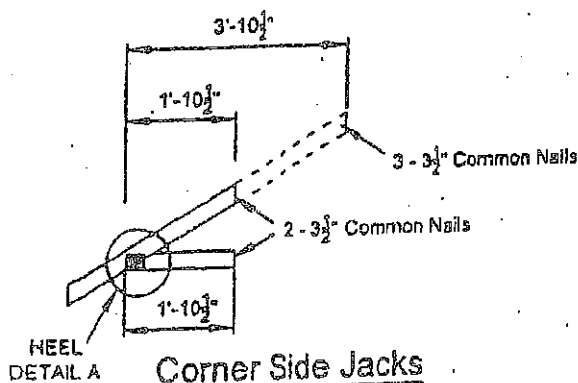
TOP CHORD DEAD LOAD : 3.0 P.S.F.

BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.

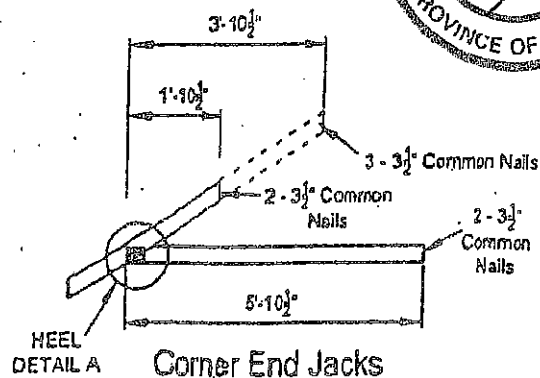
BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD : 44.8 P.S.F.

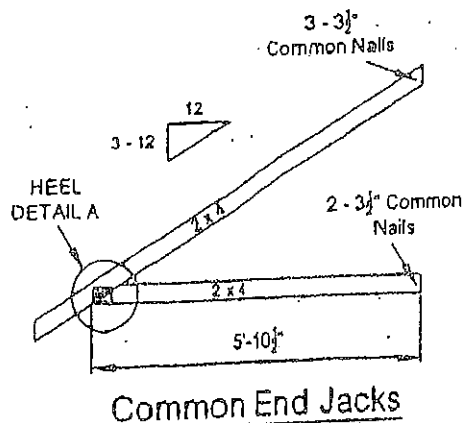
DWG NO TAN 3495.14
STRUCTURAL
COMPONENT ONLY



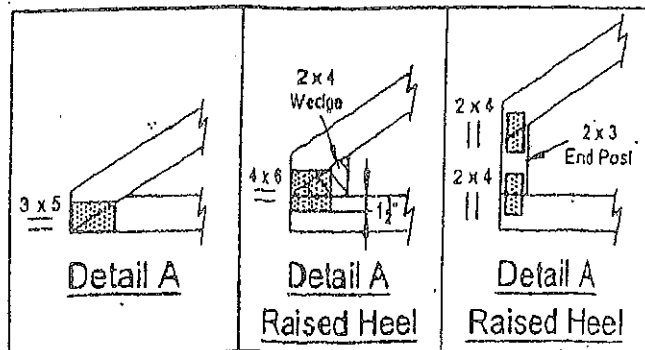
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

MICRO CITY ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

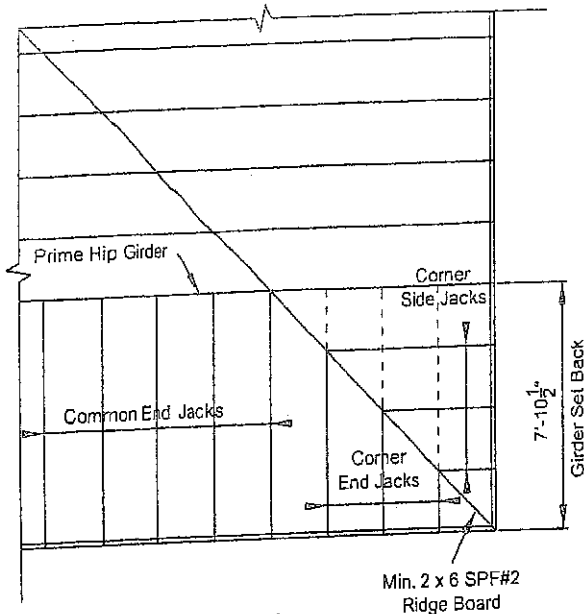
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

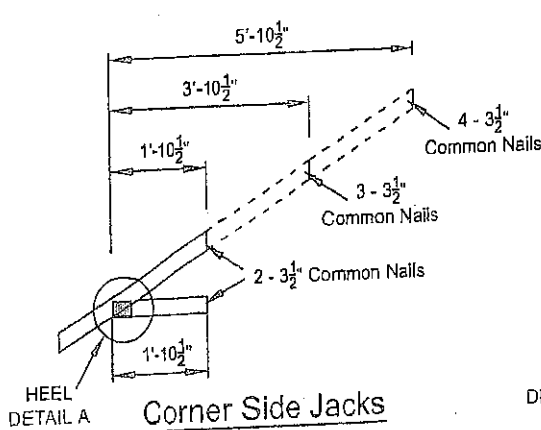
DESIGN LOAD:

TOP CHORD LIVE LOAD : 40.5 P.S.F.
TOP CHORD DEAD LOAD : 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

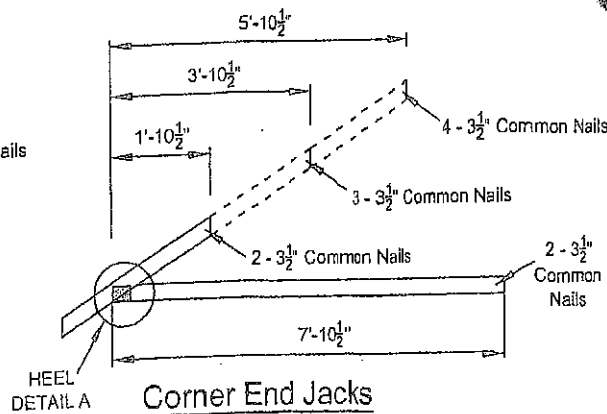
TOTAL LOAD : 50.5 P.S.F.



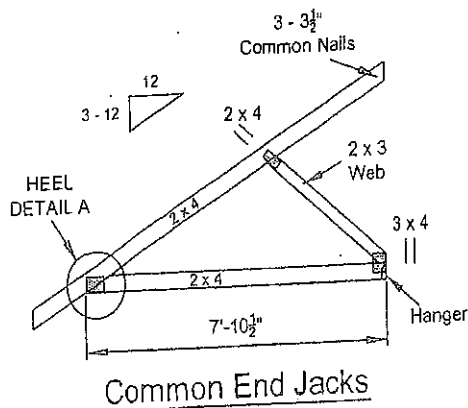
45° Hip End



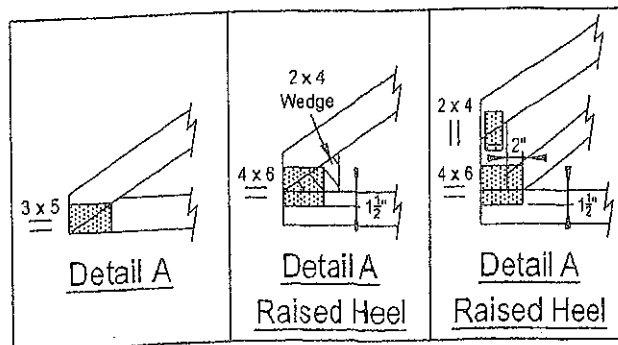
Corner Side Jacks



Corner End Jacks



Common End Jacks



DWG NO TAM 3504.1
STRUCTURAL
COMPONENT ONLY



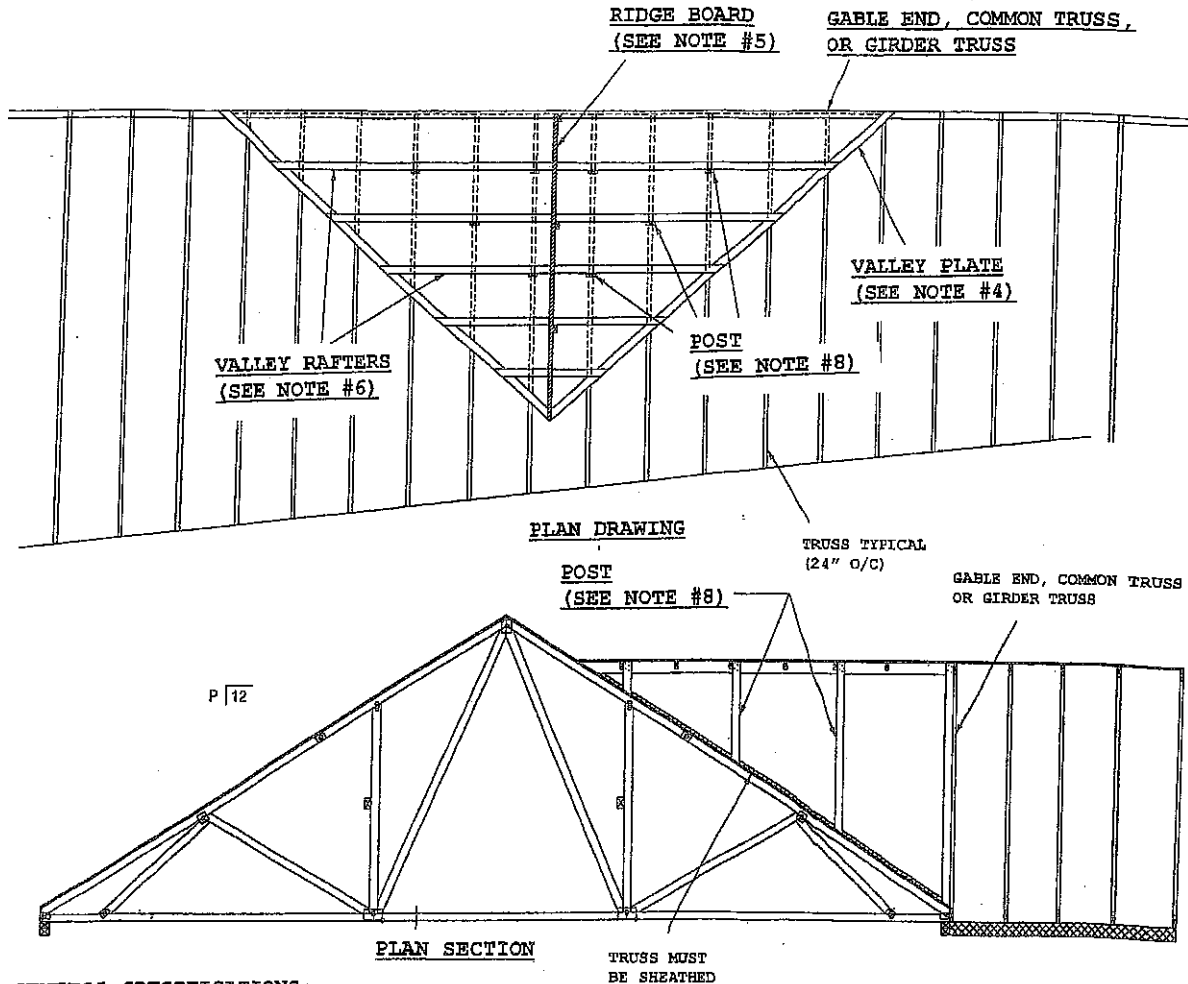
NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

MICRO CITY ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

CONVENTIONAL VALLEY FRAMING DETAIL



GENERAL SPECIFICATIONS:

- (1) WITH THE BASE TRUSSES ERECTED (INSTALLED), APPLY SHEATHING TOP CHORD OF SUPPORTING (BASE) TRUSSES.
- (2) BRACE BOTTOM CHORD AND WEB MEMBERS AS PER PRE-ENGINEERED TRUSS DESIGNS.
- (3) DEFINE VALLEY RIDGE BY RUNNING A LEVEL STRING FROM THE INTERSECTING RIDGE OF THE (a) GABLE END, (b) GIRDER TRUSS OR (c) COMMON TRUSS TO THE ROOF SHEATHING.
- (4) INSTALL 2 X 6 VALLEY PLATES ON FLAT. FASTEN TO EACH SUPPORTING TRUSS WITH (2) 16d (3.5" X 0.131") NAILS.
- (5) SET A 2 X 6 #2 RIDGE BOARD (MAX. 10'-0" RIDGE) OR 2 X 8 #2 SPF RIDGE BOARD (MAX. 20'-0" RIDGE). SUPPORT RIDGE BOARD WITH 2 X 4 POSTS SPACED 48" O/C. BEVEL BOTTOM OF POST TO SET EVENLY ON THE SHEATHING. FASTEN POST TO RIDGE WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST TO ROOF SHEATHING WITH (3) 10d (3" X 0.131") TOE-NAILS.
- (6) FRAME VALLEY RAFTERS FROM VALLEY PLATE TO RIDGE BOARD. MAXIMUM RAFTER SPACING IS 24" O/C. FASTEN VALLEY RAFTER TO RIDGE BEAM WITH (3) 16d (3.5" X 0.131") TOE-NAILS. FASTEN VALLEY RAFTER TO VALLEY PLATE WITH (3) 16d (3.5" X 0.131") TOE-NAILS.
- (7) SUPPORT THE VALLEY RAFTERS WITH 2 X 4 POSTS AT 48" O/C (OR LESS) ALONG EACH RAFTER. INSTALL POSTS IN A STAGGERED PATTERN AS SHOWN ON PLAN DRAWING. ALIGN POSTS WITH TRUSSES BELOW. FASTEN VALLEY RAFTER TO POST WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST THROUGH SHEATHING TO SUPPORTING TRUSSES WITH (2) 16d (3.5" X 0.131") NAILS.
- (8) POSTS SHALL BE 2 X 4 #2 SPF OR BETTER. POSTS EXCEEDING 75" IN HEIGHT SHALL BE INCREASED TO 4 X 4 #2 SPF, OR BETTER, OR BE PRE-ASSEMBLED TWO (2) PLY 2 X 4 #2 SPF OR BETTER FASTENED TOGETHER WITH 2 ROWS OF 10d (3" X 0.131") NAILS AT 6" O/C.
- (9) MAINTAIN A MINIMUM 3/4" LUMBER EDGE DISTANCE WHEN NAILING. NAIL SPACING SHOULD APPROXIMATE A MINIMUM 1-3/4" O/C OR MORE UNLESS NOTED OTHERWISE. ALL CONSTRUCTION TO CONFORM TO ONTARIO BUILDING CODE (CURRENT ADDITION) AT ALL TIMES.

NOTES:

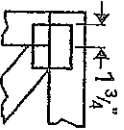
- (10) 48" O/C (MAXIMUM POST SPACING).
- (11) ROOF LIVE LOAD = 34.8 PSF (MAX.)
- (12) ROOF DEAD LOAD = 10.0 PSF (MAX.)
- (13) PART 9 APPLICATION ONLY (ONTARIO BUILDING CODE)
- (14) PART 4 APPLICATION ONLY (ONTARIO BUILDING CODE) WITH APPROVED REVIEW BY LICENSED PROFESSIONAL ENGINEER.
- (15) BASE TRUSS SPACING (24" O/C MAX.)
- (16) ALL PRE-ENGINEERED BASE TRUSS COMPONENTS TO BE SEALED BY LICENSED PROFESSIONAL ENGINEER AND THIS DETAIL TO BE VERIFIED AND APPROVED BY SAME WHEN RIDGE BOARD LENGTH EXCEEDS 12'-0".
- (17) ALL BASE TRUSSES: P = 4 (4/12) - MINIMUM.
- (18) ALL VALLEY RAFTERS: P = 4 (4/12) - MINIMUM.



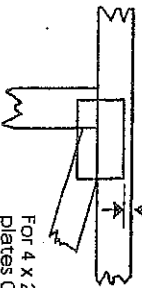
DWG NO TAM 6305.14
STRUCTURAL
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Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- $\frac{1}{4}$ " from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

* Plate location details available in Mitek software or upon request.

PLATE SIZE

4 X 4

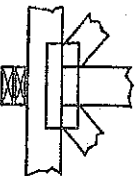
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use 1, I or Eliminator bracing if indicated.

BEARING

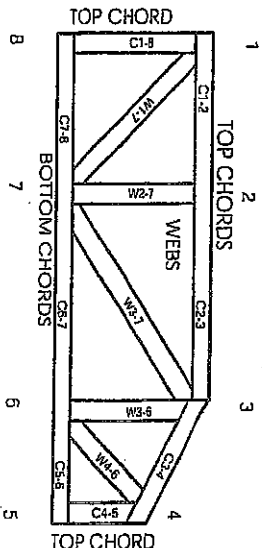


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:
TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DS8-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11998-L, 10319-L, 13270-L, 12691-R

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General Safety Notes

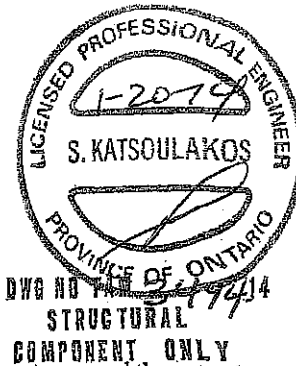
Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative I, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

Micro City Engineering Services Inc.
(BCIN: 26064; FIRM BCIN: 29991)

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Glencoe, Ontario
N0L 1M0

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Responsibilities:

Micro City Engineering Services is responsible for the design of trusses as individual components.

It is the responsibilities of others to ascertain that the design loads utilized on this (these) drawing(s) meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdiction over such decisions.

All dimensions are to be verified by the owner, contractor, architect, or other authority having input over such decisions prior to truss component manufacture. At no time shall Micro City Engineering Services Inc. or its employees be responsible for dimension errors.

Micro City Engineering Services Inc. bears no responsibility for the erection of any truss components. Persons erecting truss components are cautioned to seek professional advice regarding temporary and permanent bracing systems and to be **totally** familiar with all aspects of truss erection prior to proceeding on any truss component erection job. Any bracing shown on Micro City Engineering Services Inc. or Tamarack Roof Trusses Inc. sealed or unsealed truss component drawings is specified for the single truss component in question and is identified as an integral part of the design for that particular truss component but is **not** meant to represent the only required bracing for that particular truss component when installed as a component in a series of truss components in a roof truss system.

It is the truss manufacturer's responsibility to ensure that trusses are manufactured in accordance with Micro City Engineering Services Inc. specifications outlined below:

SPECIFICATIONS:

Truss components sealed by Micro City Engineering Services Inc. must conform to the relevant sections of the current Building Code of Ontario and Canada (Part 4 or Part 9) or the current Farm Building Code of Canada in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the Truss Plate Institute of Canada (TPIC). All unit lumber and nailing stresses identified on truss component design drawings and/or used in the design of individual truss components shall conform to the current CSA Wood Design standard identified in the current Building Code and TPIC Design Standards.

The lumber used to manufacture any truss component is to conform to the specified size and grade identified on the truss drawing.

The lumber used in the manufacture of any truss component is not to exceed 19% during its service use unless specifically noted on the truss drawing.

The lumber used in the manufacture of any truss component is not to be treated with any chemicals during its service life unless specifically noted on the truss drawing.

Connector plates shall be applied to both faces of the truss component at each joint and shall be positioned exactly as specified.

The top chord of any truss component is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the sealed truss component drawing but not exceeding 24" o/c (Part 9 design) and not exceeding 48" o/c (Part 4 or Agricultural design).

When a truss component is to be installed with no rigid ceiling attached directly to the bottom chord, then the bottom chord is to be laterally braced at intervals not exceeding 3m (or 10'-0").

All sealed or unsealed truss component drawings provided by Micro City Engineering Services Inc. Or Tamarack Roof Trusses Inc. should be read in conjunction with the following:

Warning-Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473C rev 10-'08 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer - not the truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpinc.ca and BCSI Building Component Safety Information available from the Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA, 22314.