

ASPHALT SHINGLES
12" FINISH O.H.
R.T.M.C
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:

HGUS26-2 -(XX)
LJS26DS -(V)

8/12 PITCHES (TYP.)
UNLESS NOTED

DENOTES:

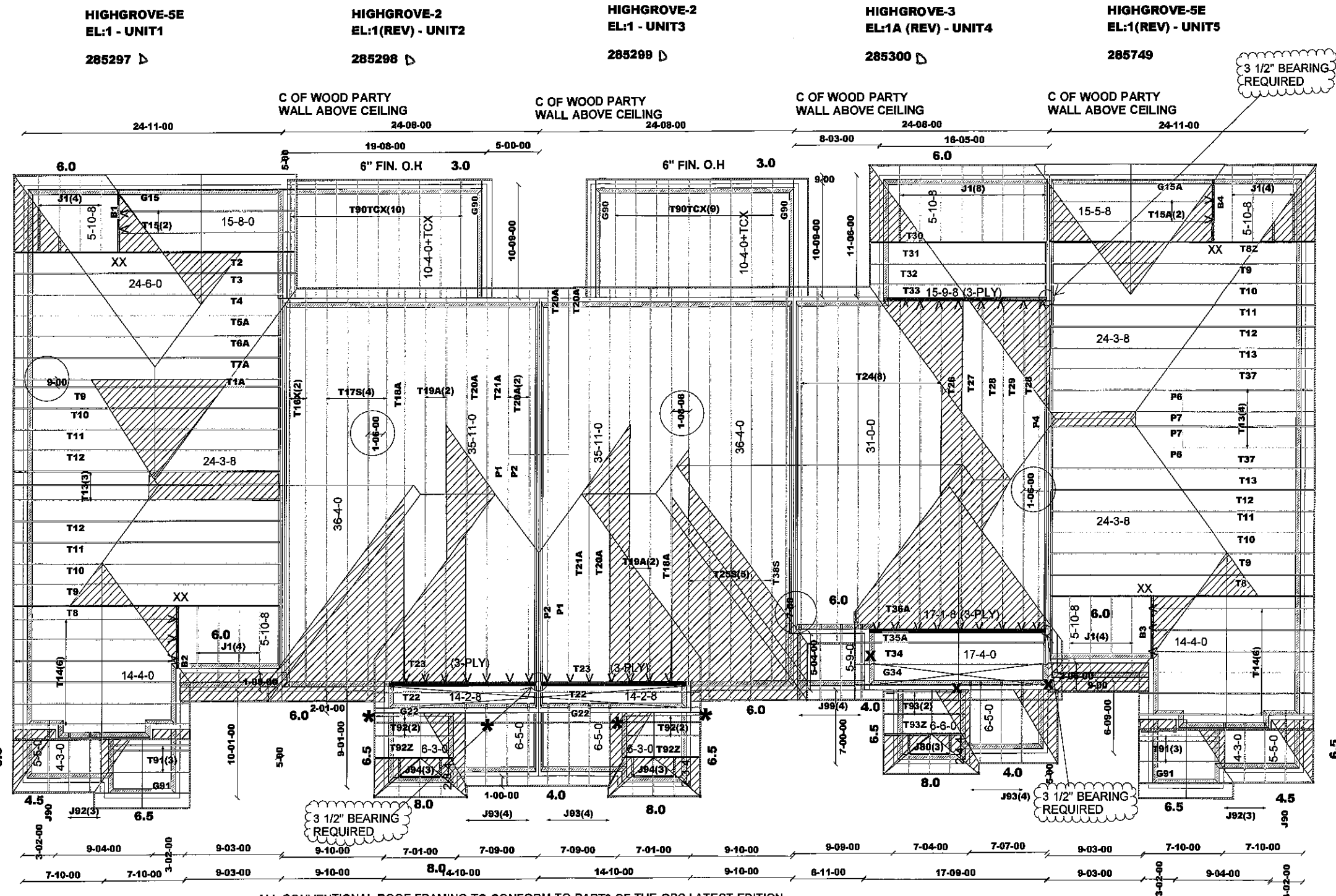
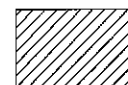
* -2'-0" HIGHER PLATE

X -1'-6" HIGHER PLATE

ALL B- 2-2X10 BEAMS (FLUSH)

T-170534

DENOTES:
CONVENTIONAL
FRAMING



ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC LATEST EDITION
ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"x4"SPF@24"o.c.
WITH A 2"x4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.
POSTS LONGER THAN 6' TO BE Laterally BRACED SO THAT THE DISTANCE
BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN LOADS:

SNOW LOAD 25.6 PSF
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

m10,994



Job Track: 43954

Layout ID: 285781

Plan Log: 93446

Builder / Location:

GREENPARK HOMES / WATERDOWN

Project: RUSSEL GARDENS PHASE 2

Date: 4/18/2018

Designer: sonny

Model / Elevation:

BLOCK206 / UNIT1TO5 (REVISED)

Milek ver 7.5.0

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TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.



Delivery Shiplist

DATE	04/18/18
SALES REP	Mario

JOB TRACK: 43954	LAYOUT ID: 285297	LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH 2	SUB-BUILDER:	
MODEL: HIGHGROVE 5E	ELEVATION: EL:1 UNIT 1	

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	T1A COMMON	8.00 0.00	24-03-08	09-06-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	102.41 64.00		
	1	T2 HIP GIRDER	8.00 0.00	24-06-00	04-01-04	2 X 6	2 X 6	01-03-08 01-01-00	01-04-13 01-04-13	145.29 89.67		
	1	T3 HIP	8.00 0.00	24-06-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	97.09 60.83		
	1	T4 HIP	8.00 0.00	24-06-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	105.79 67.00		
	1	T5A HIP	8.00 0.00	24-03-08	07-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	104.32 67.00		
	1	T6A HIP	8.00 0.00	24-03-08	08-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	108.82 69.50		
	1	T7A HIP	8.00 0.00	24-03-08	09-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	114.67 72.50		
	1	T8 HALF HIP	8.00 0.00	24-03-08	04-01-04	2 X 6	2 X 6	01-03-08 00-00-00	01-04-13 04-01-04	138.05 86.67		
	2	T9 HALF HIP	8.00 0.00	24-03-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-01-04	197.10 123.66		
	2	T10 HALF HIP	8.00 0.00	24-03-08	06-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 06-01-04	212.66 133.34		
	2	T11 HALF HIP	8.00 0.00	24-03-08	07-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 07-01-04	225.56 142.68		
	2	T12 HALF HIP	8.00 0.00	24-03-08	08-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 08-01-04	234.64 146.34		
	3	T13 HALF HIP	8.00 0.00	24-03-08	09-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 09-01-04	368.64 227.49		
	6	T14 COMMON	8.00 0.00	14-04-00	06-02-02	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	370.56 234.00		
	2	T15 COMMON	8.00 0.00	15-08-00	06-07-08	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	133.52 84.00		
	1	G15 COMMON	8.00 0.00	15-08-00	06-07-08	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	69.72 45.00		
	3	T91 COMMON	6.50 0.00	06-06-00	02-04-09	2 X 4	2 X 4	01-03-08 01-03-08	00-07-07 00-07-07	66.60 45.00		
	1	G91 COMMON	6.50 0.00	06-06-00	02-04-09	2 X 4	2 X 4	01-03-08 01-03-08	00-07-07 00-07-07	23.39 16.00		



Delivery Shiplist

DATE	04/18/18
SALES REP	Mario

JOB TRACK: 43954	LAYOUT ID: 285297	LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH2	SUB-BUILDER:	
MODEL: HIGHGROVE 5E	ELEVATION: EL:1 UNIT 1	

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	8	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	134.32 85.36		
	1	J90 MONOPITCH	4.50 0.00	05-05-00	02-05-00	2 X 4	2 X 4	01-03-08 00-00-00	00-04-10 02-05-00	14.47 9.33		
	3	J92 JACK	4.50 0.00	04-03-00	01-11-12	2 X 4	2 X 4	01-03-08 00-00-00	00-04-10 01-11-12	35.16 24.00		

TOTAL # TRUSS= 44.00

TOTAL BFT OF ALL TRUSSES=

1893.37 BFT. TOTAL WEIGHT OF ALL TRUSSES= 3002.78 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
5	Hangers	LJS26DS	

TOTAL # ITEMS= 7.00



Delivery Shiplist

DATE	08/22/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 285298 D LOCATION:
 BUILDER: GREENPARK - RUSSELL GARDENS PH 1 SUB-BUILDER:
 MODEL: HIGHGROVE 2 ELEVATION: EL:1 (REV.)- UNIT2

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	2	T16X COMMON	6.00 0.00	36-04-00	09-09-12	2 X 4	2 X 6	01-03-08 00-00-00	01-02-00 00-03-08	328.12 200.34		
	4	T17S ROOF	6.00 0.00	36-04-00	09-09-12	2 X 4	2 X 6	01-03-08 01-03-08	01-02-00 01-02-00	717.04 446.68		
	1	T18A HIP	6.00 0.00	35-11-00	09-02-08	2 X 4	2 X 6	01-03-08 00-00-00	01-02-00 01-04-08	180.38 109.67		
	2	T19A COMMON	6.00 0.00	35-11-00	10-03-00	2 X 4	2 X 6	01-03-08 00-00-00	01-02-00 01-04-08	345.82 209.34		
	3	T20A HALF HIP	6.00 0.00	35-11-00	08-04-08	2 X 4	2 X 6	01-03-08 00-00-00	01-02-00 08-04-08	570.87 345.00		
	1	T21A HALF HIP	6.00 0.00	35-11-00	09-08-08	2 X 4	2 X 6	01-03-08 00-00-00	01-02-00 09-08-08	203.45 121.67		
	1	T22 MONOPITCH	8.00 0.00	14-02-08	10-10-08	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 10-10-08	67.56 41.67		
	1	G22 MONOPITCH	8.00 0.00	14-02-08	10-10-08	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 10-10-08	77.96 48.67		
	1 3 Ply	T23 MONOPITCH	8.00 0.00	14-02-08	12-10-08	2 X 6	2 X 6	00-00-00 00-00-00	03-04-13 12-10-08	372.33 225.00		
	10	T90TCX MONOPITCH	3.00 0.00	10-04-00	03-05-09	2 X 6	2 X 4	00-09-08 00-00-00	00-08-04 03-05-09	449.90 303.30		
	1	G90 MONOPITCH	3.00 0.00	10-04-00	03-03-04	2 X 6	2 X 4	00-09-08 00-00-00	00-08-04 03-03-04	40.90 26.67		
	2	T92 COMMON	6.50 0.00	06-03-00	02-03-12	2 X 4	2 X 4	01-03-08 01-03-08	00-07-07 00-07-07	43.08 30.00		
	1	T92Z COMMON	6.50 0.00	06-03-00	02-03-12	2 X 4	2 X 4	01-03-08 01-03-08	00-07-07 00-07-07	21.54 15.00		
	1	P1 PIGGYBACK	6.00 0.00	20-06-11	02-06-08	2 X 4	2 X 4	00-00-00 00-00-00	00-00-00 02-08-00	69.01 42.67		
	1	P2 PIGGYBACK	6.00 0.00	20-06-11	03-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-00-00 04-00-00	69.87 44.00		
	4	J93 MONOPITCH	4.00 0.00	06-05-00	02-05-10	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-05-10	72.64 46.68		
	3	J94 JACK-OPEN	8.00 0.00	02-03-04	02-03-13	2 X 4	2 X 4	01-03-08 00-00-00	00-09-10 02-03-13	24.27 18.00		

TOTAL # TRUSS= 41.00

TOTAL BFT OF ALL TRUSSES=

2274.36 BFT. TOTAL WEIGHT OF ALL TRUSSES= 3654.74 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
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Delivery Shiplist

DATE	08/22/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 285298 LOCATION:
 BUILDER: GREENPARK - RUSSELL GARDENS PH 1 SUB-BUILDER:
 MODEL: HIGHGROVE 2 ELEVATION: EL:1 (REV.)- UNIT2

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
7	Hangers	LJS26DS	

TOTAL # ITEMS= 7.00



Delivery Shiplist

DATE	08/22/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 285299 D LOCATION:
 BUILDER: GREENPARK - RUSSELL GARDENS PH 1 SUB-BUILDER:
 MODEL: HIGHGROVE 2 ELEVATION: EL:1 UNIT3

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	T38S ROOF	6.00 0.00	36-04-00	08-11-00	2 X 4	2 X 6	01-03-08 00-00-00	01-02-00 01-02-00	175.30 110.33		
	5	T25S ROOF	6.00 0.00	36-04-00	08-11-00	2 X 4	2 X 6	01-03-08 01-03-08	01-02-00 01-02-00	838.65 520.85		
	1	T18A HIP	6.00 0.00	35-11-00	09-02-08	2 X 4	2 X 6	01-03-08 00-00-00	01-02-00 01-04-08	180.38 109.67		
	2	T19A COMMON	6.00 0.00	35-11-00	10-03-00	2 X 4	2 X 6	01-03-08 00-00-00	01-02-00 01-04-08	345.82 209.34		
	3	T20A HALF HIP	6.00 0.00	35-11-00	08-04-08	2 X 4	2 X 6	01-03-08 00-00-00	01-02-00 08-04-08	570.87 345.00		
	1	T21A HALF HIP	6.00 0.00	35-11-00	09-08-08	2 X 4	2 X 6	01-03-08 00-00-00	01-02-00 09-08-08	203.45 121.67		
	1	T22 MONOPITCH	8.00 0.00	14-02-08	10-10-08	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 10-10-08	67.56 41.67		
	1	G22 MONOPITCH	8.00 0.00	14-02-08	10-10-08	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 10-10-08	77.96 48.67		
	1 3 Ply	T23 MONOPITCH	8.00 0.00	14-02-08	12-10-08	2 X 6	2 X 6	00-00-00 00-00-00	03-04-13 12-10-08	372.33 225.00		
	9	T90TCX MONOPITCH	3.00 0.00	10-04-00	03-05-09	2 X 6	2 X 4	00-09-08 00-00-00	00-08-04 03-05-09	404.91 272.97		
	2	G90 MONOPITCH	3.00 0.00	10-04-00	03-03-04	2 X 6	2 X 4	00-09-08 00-00-00	00-08-04 03-03-04	81.80 53.34		
	2	T92 COMMON	6.50 0.00	06-03-00	02-03-12	2 X 4	2 X 4	01-03-08 01-03-08	00-07-07 00-07-07	43.08 30.00		
	1	T92Z COMMON	6.50 0.00	06-03-00	02-03-12	2 X 4	2 X 4	01-03-08 01-03-08	00-07-07 00-07-07	21.54 15.00		
	1	P1 PIGGYBACK	6.00 0.00	20-06-11	02-06-08	2 X 4	2 X 4	00-00-00 00-00-00	00-00-00 02-08-00	69.01 42.67		
	1	P2 PIGGYBACK	6.00 0.00	20-06-11	03-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-00-00 04-00-00	69.87 44.00		
	4	J93 MONOPITCH	4.00 0.00	06-05-00	02-05-10	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-05-10	72.64 46.68		
	3	J94 JACK-OPEN	8.00 0.00	02-03-04	02-03-13	2 X 4	2 X 4	01-03-08 00-00-00	00-09-10 02-03-13	24.27 18.00		

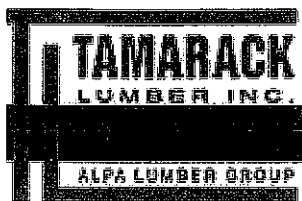
TOTAL # TRUSS= 41.00

TOTAL BFT OF ALL TRUSSES=

2254.86 BFT. TOTAL WEIGHT OF ALL TRUSSES= 3619.44 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
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Delivery Shiplist

DATE	08/22/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 285299 D LOCATION:
 BUILDER: GREENPARK - RUSSELL GARDENS PH 2 SUB-BUILDER:
 MODEL: HIGHGROVE 2 ELEVATION: EL:1 UNIT3

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
7	Hangers	LJS26DS	

TOTAL #ITEMS= 7.00



Delivery Shiplist

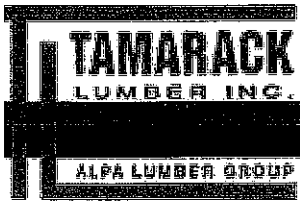
DATE	08/22/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 285300 D LOCATION:
 BUILDER: GREENPARK - RUSSELL GARDENS PH 2 SUB-BUILDER:
 MODEL: HIGHGROVE 3 ELEVATION: EL:1A(REV)- UNIT4

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	8	T24 COMMON	6.00 0.00	31-00-00	08-11-00	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	972.40 612.00		
	1	T26 ROOF	-6.00 0.00	31-00-00	06-05-02	2 X 4	2 X 4	00-00-00 00-00-00	06-05-02 02-07-00	135.01 85.33		
	1	T27 HIP	6.00 0.00	31-00-00	07-09-02	2 X 4	2 X 4	00-00-00 00-00-00	06-11-00 02-07-00	147.78 94.84		
	2	T28 HIP	6.00 0.00	31-00-00	09-01-02	2 X 4	2 X 4	00-00-00 00-00-00	06-11-00 02-07-00	301.90 189.00		
	1	T29 HIP	6.00 0.00	31-00-00	10-05-02	2 X 4	2 X 4	00-00-00 00-00-00	06-11-00 02-07-00	157.48 97.00		
	1	T34 COMMON	8.00 0.00	17-04-00	07-02-02	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	73.02 46.17		
	1	G34 COMMON	8.00 0.00	17-04-00	07-02-02	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	76.31 49.17		
	1	T35A COMMON	8.00 0.00	17-01-08	08-08-02	2 X 4	2 X 4	01-03-08 00-00-00	02-10-13 03-00-08	78.95 50.50		
	1 3 Ply	T36A COMMON	8.00 0.00	17-01-08	08-08-02	2 X 6	2 X 6	00-00-00 00-00-00	02-10-13 03-00-08	375.66 237.99		
	1	T30 HALF HIP	8.00 0.00	15-09-08	04-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-04-13 04-01-04	77.31 47.50		
	1	T31 HALF HIP	8.00 0.00	15-09-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-01-04	67.94 43.00		
	1	T32 HALF HIP	8.00 0.00	15-09-08	06-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 06-01-04	74.63 46.67		
	1 3 Ply	T33 HALF HIP	8.00 0.00	15-09-08	06-08-12	2 X 6	2 X 6	00-00-00 00-00-00	01-04-13 06-08-12	341.19 201.00		
	2	T93 COMMON	6.50 0.00	06-06-00	02-04-09	2 X 4	2 X 4	01-03-08 01-03-08	00-07-07 00-07-07	44.40 30.00		
	1	T93Z COMMON	6.50 0.00	06-06-00	02-04-09	2 X 4	2 X 4	01-03-08 01-03-08	00-07-07 00-07-07	22.20 15.00		
	1	P4 PIGGYBACK	6.00 0.00	11-08-12	02-02-08	2 X 4	2 X 4	00-00-00 00-00-00	00-00-00 00-00-00	36.23 23.83		
	8	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	134.32 85.36		
	3	J80 JACK-OPEN	8.00 0.00	02-04-04	02-04-08	2 X 4	2 X 4	01-03-08 00-00-00	00-09-10 02-04-08	24.93 18.00		



Delivery Shiplist

DATE	08/22/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 285300 D LOCATION:
 BUILDER: GREENPARK - RUSSELL GARDENS PH2 SUB-BUILDER:
 MODEL: HIGHGROVE 3 ELEVATION: EL:1A(REV)- UNIT4

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	4	J93 MONOPITCH	4.00 0.00	06-05-00	02-05-10	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-05-10	72.64 46.68		
	4	J99 MONOPITCH	4.00 0.00	05-09-00	02-02-15	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-02-15	60.52 37.32		

TOTAL # TRUSS= 48.00

TOTAL BFT OF ALL TRUSSES=

2056.36 BFT. TOTAL WEIGHT OF ALL TRUSSES= 3274.82 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
17	Hangers	LJS26DS	

TOTAL # ITEMS= 17.00



Delivery Shiplist

DATE	04/18/18
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 285749 LOCATION:
 BUILDER: GREENPARK - RUSSELL GARDENS PH2 SUB-BUILDER:
 MODEL: HIGHGROVE 5E ELEVATION: 1(REV)- UNIT5

ROOF TRUSSES

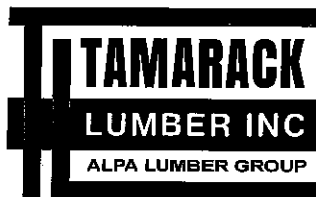
ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	T8 HALF HIP	8.00 0.00	24-03-08	04-01-04	2 X 6	2 X 6	01-03-08 00-00-00	01-04-13 04-01-04	143.26 88.34		
	1	T8Z HALF HIP	8.00 0.00	24-03-08	04-01-04	2 X 6	2 X 6	01-03-08 00-00-00	01-04-13 04-01-04	143.26 88.34		
	2	T9 HALF HIP	8.00 0.00	24-03-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-01-04	197.10 123.66		
	2	T10 HALF HIP	8.00 0.00	24-03-08	06-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 06-01-04	212.66 133.34		
	2	T11 HALF HIP	8.00 0.00	24-03-08	07-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 07-01-04	225.56 142.68		
	2	T12 HALF HIP	8.00 0.00	24-03-08	08-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 08-01-04	234.64 146.34		
	6	T13 HALF HIP	8.00 0.00	24-03-08	09-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 09-01-04	737.28 454.98		
	2	T37 HALF HIP	8.00 0.00	24-03-08	10-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 10-01-04	243.36 152.00		
	2	T15A COMMON	8.00 0.00	15-05-08	06-07-08	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	129.00 82.66		
	1	G15A COMMON	8.00 0.00	15-05-08	06-07-08	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	67.52 44.33		
	6	T14 COMMON	8.00 0.00	14-04-00	06-02-02	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	370.56 234.00		
	3	T91 COMMON	6.50 0.00	06-06-00	02-04-09	2 X 4	2 X 4	01-03-08 01-03-08	00-07-07 00-07-07	66.60 45.00		
	1	G91 COMMON	6.50 0.00	06-06-00	02-04-09	2 X 4	2 X 4	01-03-08 01-03-08	00-07-07 00-07-07	23.39 16.00		
	2	P6 PIGGYBACK	8.00 0.00	11-11-15	01-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-00-00 02-00-00	80.22 51.34		
	2	P7 PIGGYBACK	8.00 0.00	11-11-15	02-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-00-00 03-00-00	85.52 56.00		
	8	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	134.32 85.36		
	1	J90 MONOPITCH	4.50 0.00	05-05-00	02-05-00	2 X 4	2 X 4	01-03-08 00-00-00	00-04-10 02-05-00	14.47 9.33		
	3	J92 MONOPITCH	4.50 0.00	04-03-00	01-11-12	2 X 4	2 X 4	01-03-08 00-00-00	00-04-10 01-11-12	35.16 24.00		

TOTAL # TRUSS= 47.00

TOTAL BFT OF ALL TRUSSES=

1977.70 BFT. TOTAL WEIGHT OF ALL TRUSSES= 3143.88 LBS.



Delivery Shiplist

DATE	04/18/18
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 285749 LOCATION:
 BUILDER: GREENPARK - RUSSELL GARDENS PH 2 SUB-BUILDER:
 MODEL: HIGHGROVE 5E ELEVATION: 1(REV)- UNIT5

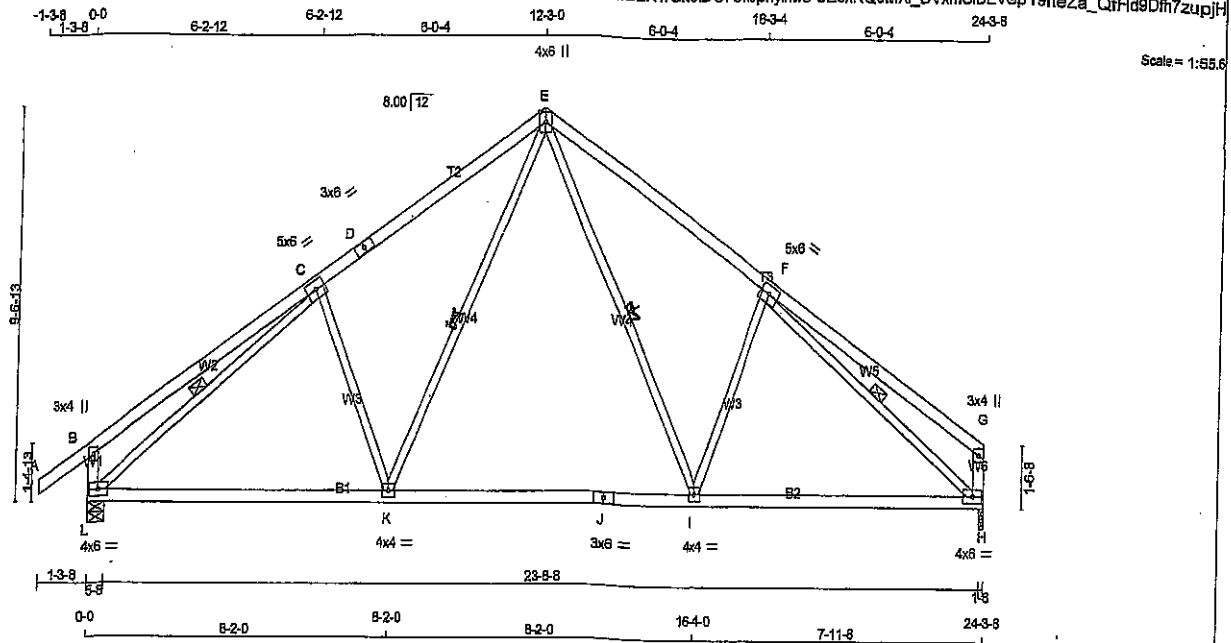
HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
5	Hangers	LJS26DS	

TOTAL # ITEMS= 7.00

JOB NAME 285994	TRUSS NAME T1A	QUANTITY 1	PLY 1	JOB DESC. 43954	DRWG NO.
Tamerack Roof Truss, Burlington					

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ID:w3K2IB2K4fGk9DO7SkJphylMC-oE6xRQ0Wxi_DVxmSIBEv19heZa_QfhId9Dfh7zupJH



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
L - B	2x4	DRY	No.2	SPF	
H - G	2x4	DRY	No.2	SPF	
L - J	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0			
C	TMWW-1	MT20	5.0	3.0			
D	TS-1	MT20	3.0	6.0			
E	TTWW+p	MT20	4.0	6.0	Edge		
F	TMWW-1	MT20	5.0	6.0			
G	TMV+p	MT20	3.0	4.0			
H	BMWW-1	MT20	4.0	6.0			
I	BMWW-1	MT20	4.0	4.0			
J	BS-1	MT20	3.0	6.0			
K	BMWW-1	MT20	4.0	4.0			
L	BMWW-1	MT20	4.0	6.0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
		GROSS REACTION		GROSS REACTION		BRG		BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX		
L	1479	0	1479	0	5-8	5-8			
H	1363	0	1363	0	0	1-8	1-8		

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN. COMPONENT REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
L	1198	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0		
H	1119	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.96 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-L F.H. *ER, E, I, J*

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CS1 (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CS1 (LC)
FR-TO		FROM	TO		FR-TO		
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	C-K	-352 / 44 0.19 (1)
B-C	0 / 34	-84.3	-84.3	0.52 (1)	10.00	K-E	0 / 632 0.14 (1)
C-D	-1409 / 0	-84.3	-84.3	0.45 (1)	4.98	E-I	0 / 599 0.13 (1)
D-E	-1409 / 0	-84.3	-84.3	0.45 (1)	4.98	I-F	-318 / 58 0.17 (1)
E-F	-1388 / 0	-84.3	-84.3	0.44 (1)	5.01	L-C	-1669 / 0 0.71 (1)
F-G	0 / 34	-84.3	-84.3	0.50 (1)	10.00	F-H	-1653 / 0 0.67 (1)
L-B	-311 / 0	0.0	0.0	0.03 (1)	7.61		
H-G	-186 / 0	0.0	0.0	0.02 (1)	7.61		
L-K	0 / 1275	-28.0	-28.0	0.58 (2)	10.00		
K-J	0 / 689	-28.0	-28.0	0.54 (2)	10.00		
J-I	0 / 889	-28.0	-28.0	0.54 (2)	10.00		
I-H	0 / 1245	-28.0	-28.0	0.58 (2)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.8	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/380$ (0.81")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.17")
ALLOWABLE DEFL.(TL) = $L/380$ (0.81")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.29")

CS1: TC=0.52/1.00 (B-C:1), BC=0.58/1.00 (K-L:2),
WB=0.71/1.00 (C-L:1), SS=0.21/1.00 (C-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PL)
	MAX MIN	MAX MIN
MT20	618 354	1687 622
	2284	1656

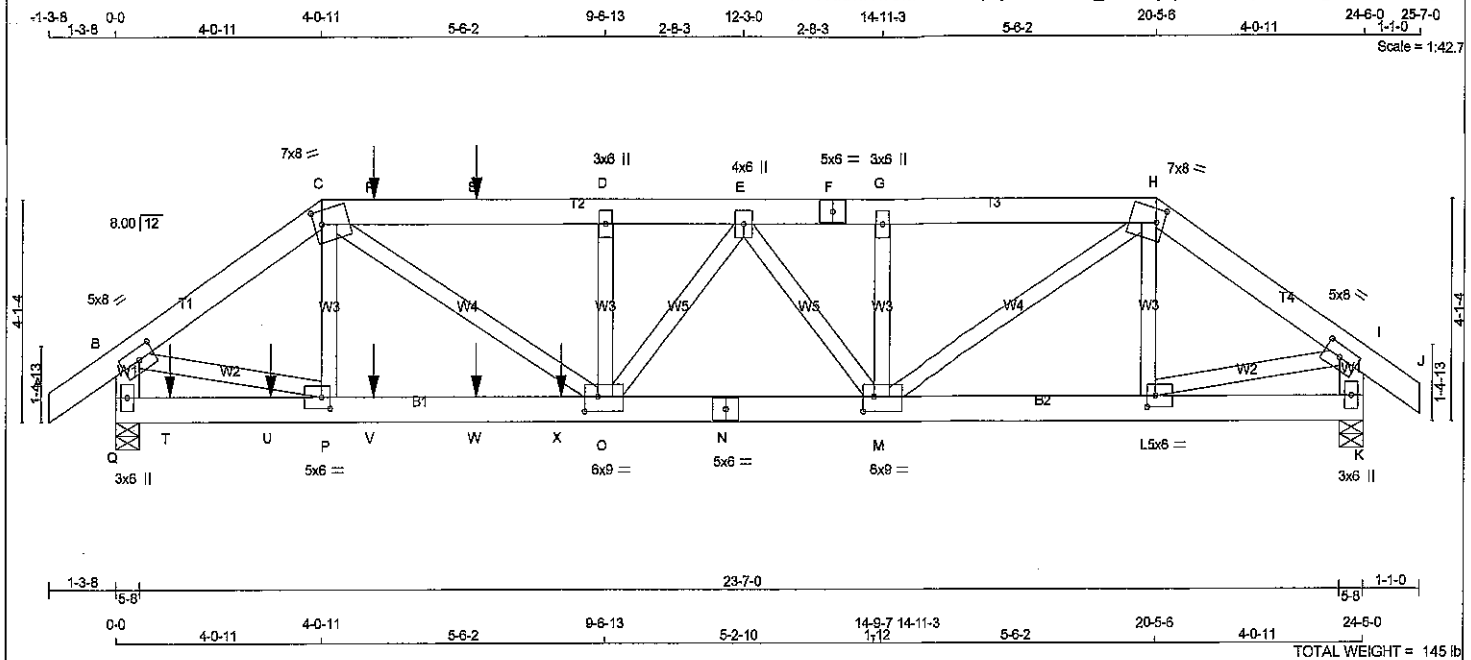
PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (L) (INPUT = 0.90)
JSI METAL= 0.43 (C) (INPUT = 1.00)



DWG NO. TAM3984-18
STRUCTURAL
COMPONENT ONLY



LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - C	2x6	DRY	No.2	SFF
C - F	2x6	DRY	No.2	SFF
F - H	2x6	DRY	No.2	SFF
H - J	2x6	DRY	No.2	SFF
K - B	2x6	DRY	No.2	SFF
Q - I	2x6	DRY	No.2	SFF
Q - N	2x6	DRY	No.2	SFF
N - K	2x6	DRY	No.2	SFF

ALL WEBS 2x4 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.50	3.75
C	TTWW-m	MT20	7.0	8.0	3.00	1.75
D	TMW+w	MT20	3.0	6.0		
E	TMWW+t	MT20	4.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMW+w	MT20	3.0	6.0		
H	TTWW-m	MT20	7.0	8.0	3.00	1.75
I	TMVW-t	MT20	5.0	8.0	2.50	3.75
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.00
M	BMWWWW-t	MT20	6.0	9.0	3.25	2.50
N	BS-t	MT20	5.0	6.0		
O	BMWWWW-t	MT20	6.0	9.0	3.25	3.00
P	BMWW-t	MT20	5.0	6.0	2.50	2.00
Q	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 101.4 lbs FACTORED DOWN AT 5-0-12, AND 101.4 lbs FACTORED DOWN AT 7-0-12 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 1-0-12, 69.9 lbs FACTORED DOWN AT 3-0-12, 69.9 lbs FACTORED DOWN AT 5-0-12, AND 69.9 lbs FACTORED DOWN AT 7-0-12, AND 1210.8 lbs FACTORED DOWN AT 8-8-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER								
BEARINGS			FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
JT	VERT	HORIZ	DOWN	HORIZ	UPLIFT			
Q	2561	0	2561	0	0	5-8		5-8
K	1984	0	1984	0	0	5-8		5-8

UNFACTORED REACTIONS

1ST CASE		MAX/MIN COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	2104	1168 / 0	481 / 0	0 / 0	0 / 0	457 / 0	0 / 0
K	1609	925 / 0	345 / 0	0 / 0	0 / 0	339 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.70 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED			
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	CL1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO		FR-TO		
A-B	0 / 33	-84.3	-84.3	0.07 (1)	10.00	P-C	-68 / 266 0.05 (3)
B-C	-3048 / 0	-84.3	-84.3	0.18 (1)	4.65	C-O	0 / 2284 0.40 (1)
C-R	-4436 / 0	-84.3	-84.3	0.43 (1)	3.70	O-D	-541 / 0 0.09 (1)
R-S	-4436 / 0	-84.3	-84.3	0.43 (1)	3.70	M-G	-442 / 0 0.08 (1)
S-D	-4436 / 0	-84.3	-84.3	0.43 (1)	3.70	M-H	0 / 1998 0.35 (1)
D-E	-4438 / 0	-84.3	-84.3	0.27 (1)	3.88	L-H	-214 / 76 0.04 (1)
E-F	-3550 / 0	-84.3	-84.3	0.19 (1)	4.36	B-P	0 / 2586 0.46 (1)
F-G	-3550 / 0	-84.3	-84.3	0.19 (1)	4.36	L-I	0 / 1933 0.34 (1)
G-H	-3549 / 0	-84.3	-84.3	0.27 (1)	4.28	O-E	0 / 721 0.13 (1)
H-I	-2278 / 0	-84.3	-84.3	0.16 (1)	5.25	E-M	-774 / 0 0.18 (1)
I-J	0 / 28	-84.3	-84.3	0.05 (1)	10.00		
Q-B	-2531 / 0	0.0	0.0	0.16 (1)	6.47		
K-I	-1947 / 0	0.0	0.0	0.14 (1)	7.18		
Q-T	0 / 0	-28.0	-28.0	0.23 (2)	10.00		
T-U	0 / 0	-28.0	-28.0	0.23 (2)	10.00		
U-P	0 / 0	-28.0	-28.0	0.23 (2)	10.00		
P-V	0 / 2527	-28.0	-28.0	0.77 (1)	10.00		
V-W	0 / 2527	-28.0	-28.0	0.77 (1)	10.00		
W-X	0 / 2527	-28.0	-28.0	0.77 (1)	10.00		
X-O	0 / 2527	-28.0	-28.0	0.77 (1)	10.00		
O-N	0 / 4008	-28.0	-28.0	0.79 (1)	10.00		
N-M	0 / 4008	-28.0	-28.0	0.79 (1)	10.00		
M-L	0 / 1982	-28.0	-28.0	0.33 (1)	10.00		
L-K	0 / 0	-28.0	-28.0	0.10 (2)	10.00		

LICENSED PROFESSIONAL ENGINEER

957

S. KATSOULAKOS

PROVINCIAL ENGINEER

FACTORED CONCENTRATED LOADS (LBS)							
JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE
R	5-0-12	-101	-101	—	BACK	VERT	TOTAL
S	7-0-12	-101	-101	—	BACK	VERT	TOTAL
T	1-0-12	-40	-70	—	BACK	VERT	TOTAL
U	3-0-12	-40	-70	—	BACK	VERT	TOTAL
V	5-0-12	-40	-70	—	BACK	VERT	TOTAL
W	7-0-12	-40	-70	—	BACK	VERT	TOTAL
X	8-8-8	-1211	-1211	—	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:			
TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD =		46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 201
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.82")
CALCULATED VERT. DEFL.(LL)= L/999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.82")
CALCULATED VERT. DEFL.(TL)= L/999 (0.23")

CSI: TC=0.43 (C-D:1), BC=0.79 (M-O:1),
WB=0.46 (B-P:1), SSI=0.75 (O-P:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

	NAIL VALUES		SHEAR (PLI)	SECTION (PLI)
	GRIP(DRY) (PSI)			
	MAX	MIN	MAX	MIN
MT20	618	354	1867	822
	2284	1656		

PLATE PLACEMENT TOL. = 0.250 inches

· PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (O) (INPUT = 0.90)
JSI METAL= 0.65 (N) (INPUT = 1.00)

DWGNB.TAM 44843.17

STRUCTURAL

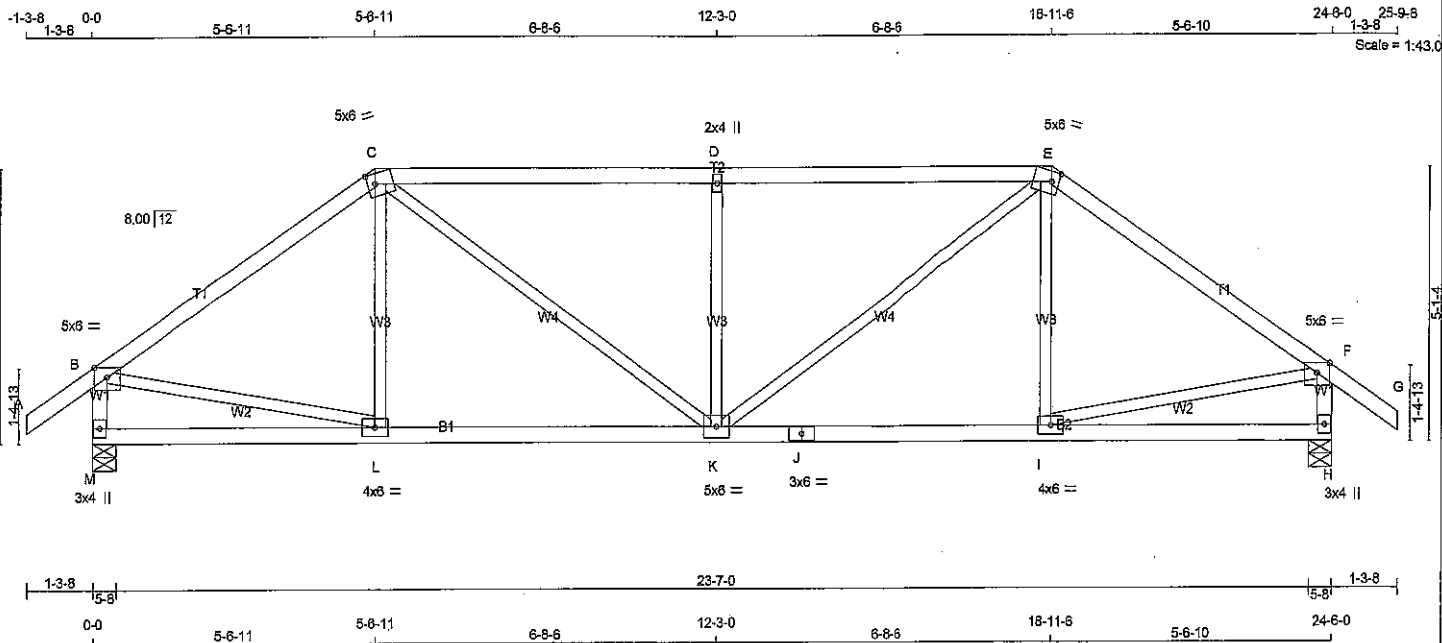
CONFIDENTIAL ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
285781	T3	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 97 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
M - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
M - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMVW-p	MT20	5.0	6.0 Edge
C	TTWW-m	MT20	5.0	6.0 2.25 1.75
D	TMVW-w	MT20	2.0	4.0
E	TTWW-m	MT20	5.0	6.0 2.25 1.75
F	TMVW-p	MT20	5.0	6.0 Edge
H	BMV1+p	MT20	3.0	4.0
I	BMVW-t	MT20	4.0	6.0
J	BS-t	MT20	3.0	6.0
K	BMVW-w	MT20	5.0	6.0
L	BMVW-t	MT20	4.0	6.0
M	BMV1+p	MT20	3.0	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
M	1491	0	1491	0	0	5-8	5-8	5-8	5-8
H	1491	0	1491	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX	MIN	COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
M	1208	697 / 0	257 / 0	0 / 0	0 / 0	253 / 0	0 / 0	0 / 0	0 / 0	0 / 0
H	1208	697 / 0	257 / 0	0 / 0	0 / 0	253 / 0	0 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.85 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)		
FR-TO		FROM	TO		FR-TO				
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	L-C	-47 / 210 0.05 (3)		
B-C	-1511 / 0	-84.3	-84.3	0.55 (1)	4.67	C-K	0 / 747 0.17 (1)		
C-D	-1862 / 0	-84.3	-84.3	0.80 (1)	3.85	K-D	-892 / 0 0.27 (1)		
D-E	-1862 / 0	-84.3	-84.3	0.80 (1)	3.85	K-E	0 / 747 0.17 (1)		
E-F	-1511 / 0	-84.3	-84.3	0.55 (1)	4.67	I-E	-47 / 210 0.05 (3)		
F-G	0 / 32	-84.3	-84.3	0.11 (1)	10.00	B-L	0 / 1278 0.29 (1)		
M-B	-1430 / 0	0.0	0.0	0.15 (1)	6.83	I-F	0 / 1278 0.29 (1)		
H-F	-1430 / 0	0.0	0.0	0.15 (1)	6.83				
M-L	0 / 0	-28.0	-28.0	0.25 (3)	10.00				
L-K	0 / 1255	-28.0	-28.0	0.40 (2)	10.00				
K-J	0 / 1255	-28.0	-28.0	0.40 (2)	10.00				
J-I	0 / 1255	-28.0	-28.0	0.40 (2)	10.00				
I-H	0 / 0	-28.0	-28.0	0.25 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, BCRC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.82")
CALCULATED VERT. DEFL.(LL)= L/999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.82")
CALCULATED VERT. DEFL.(TL)= L/999 (0.14")

CSI: TC=0.80 (C-D:1), BC=0.40 (K-L:2), WB=0.29 (B-L:1), SSI=0.27 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1067 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (L) (INPUT = 0.80)
JSI METAL= 0.37 (J) (INPUT = 1.00)



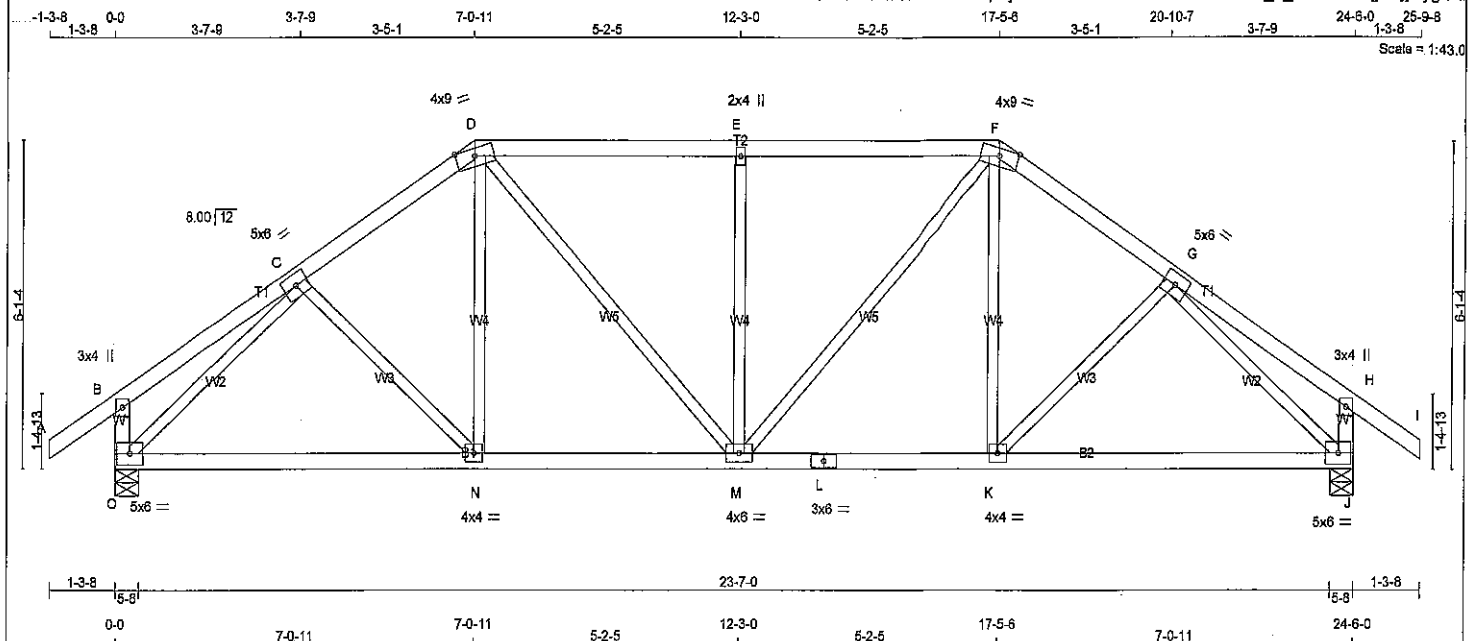
DRWG NO. TAM 4484417
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
285781	T4	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
O - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
O - L	2x4	DRY	No.2
L - J	2x4	DRY	No.2

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW-m	MT20	4.0	9.0	Edge	
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	4.0	9.0	Edge	
G	TMWW-t	MT20	5.0	6.0		
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	5.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMVW1-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
O	1491	0	1491	0
J	1491	0	1491	0

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
O	1208	697 / 0	257 / 0	0 / 0	0 / 0	253 / 0	0 / 0
J	1208	697 / 0	257 / 0	0 / 0	0 / 0	253 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.02 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CS (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)	
FR-TO		FROM TO			FR-TO			
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	C-N	0 / 119	0.03 (3)
B-C	0 / 17	-84.3	-84.3	0.15 (1)	10.00	N-D	0 / 304	0.07 (2)
C-D	-1478 / 0	-84.3	-84.3	0.14 (1)	5.27	D-M	0 / 424	0.10 (1)
D-E	-1498 / 0	-84.3	-84.3	0.32 (1)	5.02	M-E	-533 / 0	0.31 (1)
E-F	-1498 / 0	-84.3	-84.3	0.32 (1)	5.02	M-F	0 / 424	0.10 (1)
F-G	-1478 / 0	-84.3	-84.3	0.14 (1)	5.27	K-F	0 / 304	0.07 (2)
G-H	0 / 17	-84.3	-84.3	0.15 (1)	10.00	K-G	0 / 119	0.03 (3)
H-I	0 / 32	-84.3	-84.3	0.11 (1)	10.00	O-C	-1688 / 0	0.65 (1)
O-B	-233 / 0	0.0	0.0	0.02 (1)	7.81	G-J	-1688 / 0	0.65 (1)
J-H	-233 / 0	0.0	0.0	0.02 (1)	7.81			
O-N	0 / 1208	-28.0	-28.0	0.45 (2)	10.00			
N-M	0 / 1216	-28.0	-28.0	0.47 (2)	10.00			
M-L	0 / 1216	-28.0	-28.0	0.47 (2)	10.00			
L-K	0 / 1216	-28.0	-28.0	0.47 (2)	10.00			
K-J	0 / 1208	-28.0	-28.0	0.45 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.82")
CALCULATED VERT. DEFL.(LL)= L/999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.82")
CALCULATED VERT. DEFL.(TL)= L/999 (0.19")

CSI: TC=0.32 (D-E:1), BC=0.47 (M-N:2), WB=0.65 (C-O:1), SSI=0.21 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

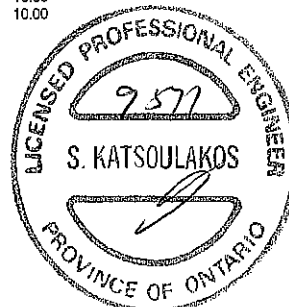
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX MIN
MT20	618	354	1987 822 2284 1856

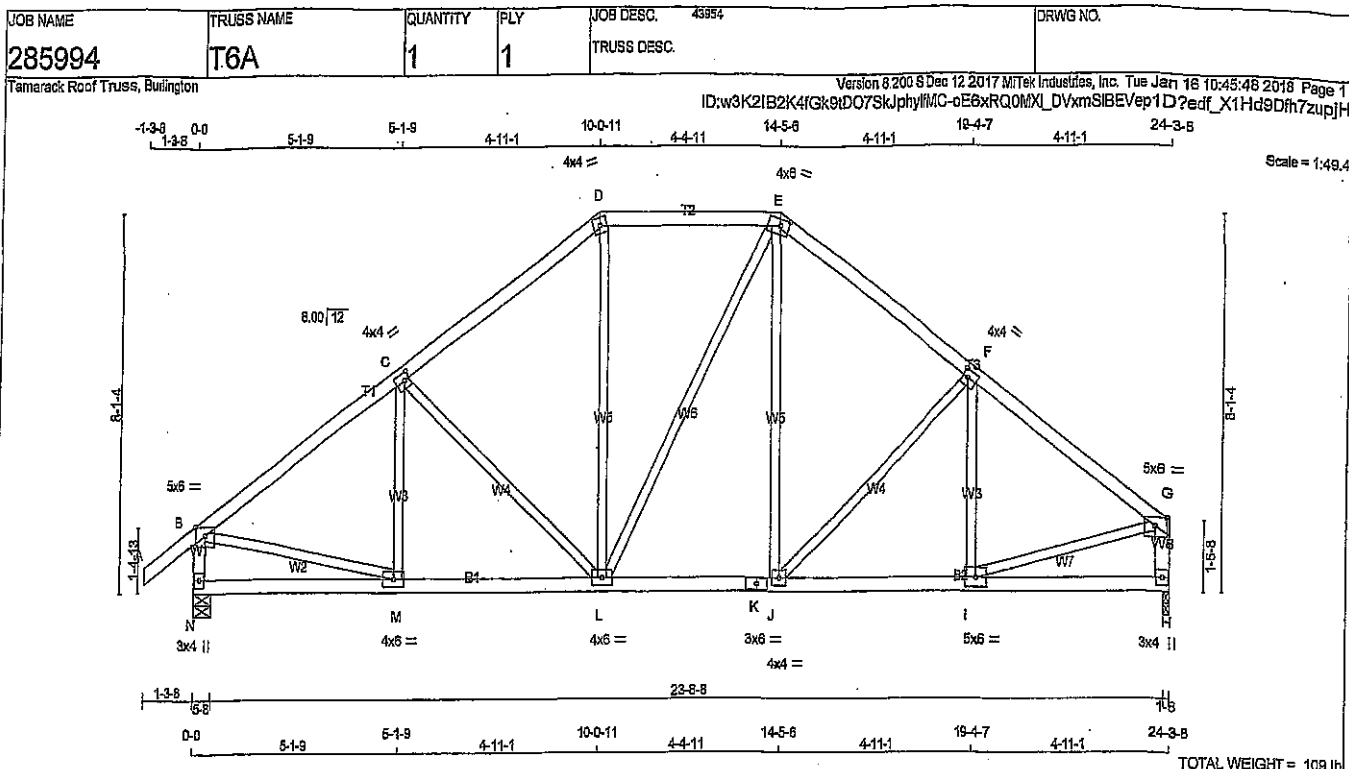
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (F) (INPUT = 0.90)
JSI METAL= 0.41 (C) (INPUT = 1.00)



DWG NO. TAM 44845-17
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TTW-m	MT20	4.0	4.0		
E	TTWV-m	MT20	4.0	6.0	1.75	2.50
F	TMVW-t	MT20	4.0	4.0	2.00	1.50
G	TMVW-p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMVW-t	MT20	5.0	6.0		
J	BMVW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMVW-t	MT20	4.0	6.0		
M	BMVW-t	MT20	4.0	6.0		
N	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
N	1479	0	1479	0	5-8	5-8
H	1363	0	1363	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST CASE		MAX/MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
N	1198	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0
H	1119	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	FACTORED HORZ. LOAD (LBS)	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 32	-84.3	-84.3 0.11 (1)	10.00	M-C	-115 / 147	0.04 (1)
B-C	-1531 / 0	-84.3	-84.3 0.31 (1)	5.00	C-L	-354 / 0	0.30 (1)
C-D	-1271 / 0	-84.3	-84.3 0.29 (1)	5.39	L-D	0 / 374	0.08 (2)
D-E	-1037 / 0	-84.3	-84.3 0.22 (1)	5.91	E-L	0 / 15	0.00 (1)
E-F	-1262 / 0	-84.3	-84.3 0.28 (1)	5.42	J-E	0 / 360	0.08 (2)
F-G	-1483 / 0	-84.3	-84.3 0.26 (1)	5.08	J-F	-310 / 0	0.28 (1)
N-B	-1420 / 0	0.0	0.0 0.16 (1)	6.84	I-F	-195 / 122	0.05 (1)
H-G	-1307 / 0	0.0	0.0 0.14 (1)	7.06	B-M	0 / 1325	0.30 (1)
					I-G	0 / 1284	0.28 (1)
N-M	0 / 0	-28.0	-28.0 0.18 (3)	10.00			
M-L	0 / 1286	-28.0	-28.0 0.32 (2)	10.00			
L-K	0 / 1030	-28.0	-28.0 0.23 (2)	10.00			
K-J	0 / 1030	-28.0	-28.0 0.23 (2)	10.00			
J-I	0 / 1257	-28.0	-28.0 0.31 (2)	10.00			
I-H	0 / 0	-28.0	-28.0 0.17 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TFC 2011

(55% OF 81.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/599 (0.08")

CSI: TO=0.31/1.00 (S-C:1), BC=0.32/1.00 (L-M:2), WB=0.30/1.00 (C-L:1), CSI=0.17/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

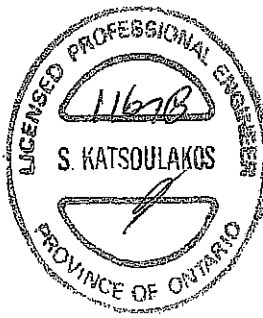
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (FBI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TDL = 0.250 inches

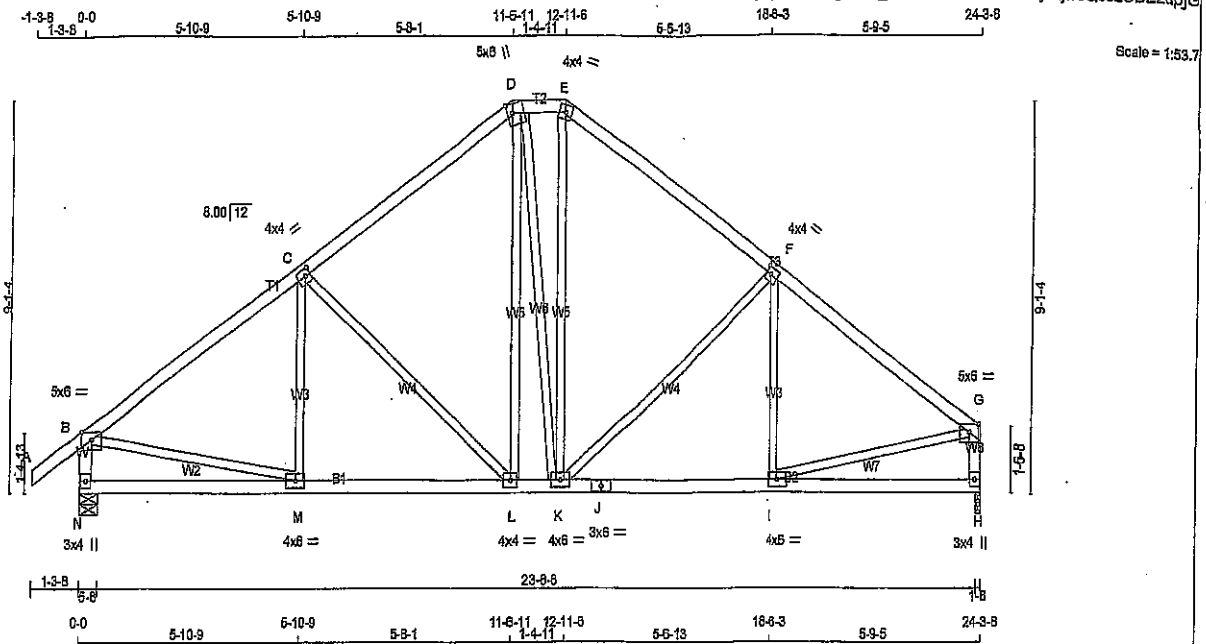
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.89 (M) (INPUT = 0.90)
JSI METAL = 0.92 (M) (INPUT = 1.00)



DRWG NO. TAM 3986-18
STRUCTURAL
COMPONENT ONLY

JOB NAME 285994	TRUSS NAME T7A	QUANTITY 1	PLY 1	JOB DESC. 43954	DRWG NO.
Tamarack Roof Truss, Burlington					



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESOR.
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
N - B	2x4	DRY	No.2
H - G	2x4	DRY	No.2
N - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMVW-p	MT20	5.0	6.0 Edge
C	TMVW-t	MT20	4.0	4.0 2.00 1.50
D	TTVW+m	MT20	5.0	6.0 2.50 1.50
E	TTVW-m	MT20	4.0	4.0
F	TMVW-t	MT20	4.0	4.0 2.00 1.50
G	TMVW-p	MT20	5.0	6.0 Edge
H	BMV1+p	MT20	3.0	4.0
I	BMVW-t	MT20	4.0	6.0
J	BS-t	MT20	3.0	6.0
K	BMVW-t	MT20	4.0	6.0
L	BMVW-t	MT20	4.0	4.0
M	BMVW-t	MT20	4.0	6.0
N	BMV1+p	MT20	3.0	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT HORZ	DOWN HORZ UPLIFT	IN-SX	IN-SX
N 1479 0	1479 0 0	5-5	5-5
H 1353 0	1353 0 0	1-8	1-8

UNFACTORED REACTIONS

1ST LOASE	MAX. MIN. COMPONENT REACTIONS						
JT COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	
N 1198	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0	
H 1119	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)
FR-TO		FROM TO		FR-TO		FROM TO	
A-B	0 / 32	-84.3 -84.3	0.11 (1)	10.00	M-C	-50 / 214	0.05 (3)
B-C	-1533 / 0	-84.3 -84.3	0.42 (1)	4.66	C-L	-481 / 0	0.57 (1)
C-D	-1163 / 0	-84.3 -84.3	0.38 (1)	5.44	L-D	0 / 377	0.08 (1)
D-E	-945 / 0	-84.3 -84.3	0.03 (1)	6.25	D-K	0 / 17	0.00 (2)
E-F	-1164 / 0	-84.3 -84.3	0.37 (1)	5.46	K-E	0 / 390	0.09 (1)
F-G	-1491 / 0	-84.3 -84.3	0.39 (1)	4.94	K-F	-433 / 0	0.51 (1)
N-B	-1413 / 0	0.0 0.0	0.15 (1)	8.66	I-F	-94 / 185	0.04 (3)
H-G	-1299 / 0	0.0 0.0	0.14 (1)	7.08	B-M	0 / 1324	0.30 (1)
				I-G	0 / 1288	0.29 (1)	
N-M	0 / 0	-28.0 -28.0	0.25 (3)	10.00			
M-L	0 / 1302	-28.0 -28.0	0.40 (2)	10.00			
L-K	0 / 943	-28.0 -28.0	0.28 (2)	10.00			
K-J	0 / 1288	-28.0 -28.0	0.37 (2)	10.00			
J-I	0 / 1288	-28.0 -28.0	0.37 (2)	10.00			
I-H	0 / 0	-28.0 -28.0	0.23 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6	PSF
DL	= 3.0	PSF
BOT CH. LL	= 10.5	PSF
DL	= 7.0	PSF
TOTAL LOAD	= 48.1	PSF

SPACING = 24.0 IN. G/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2310

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 058-09
- TFC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CSI: TC=0.42/1.00 (B-C:1), BC=0.40/1.00 (L-M:2), WB=0.57/1.00 (C-L:1), SI=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

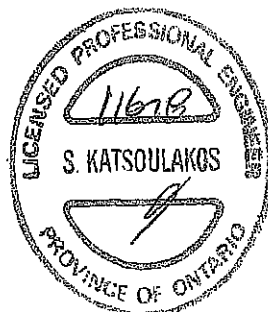
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1887 822 2284 1665

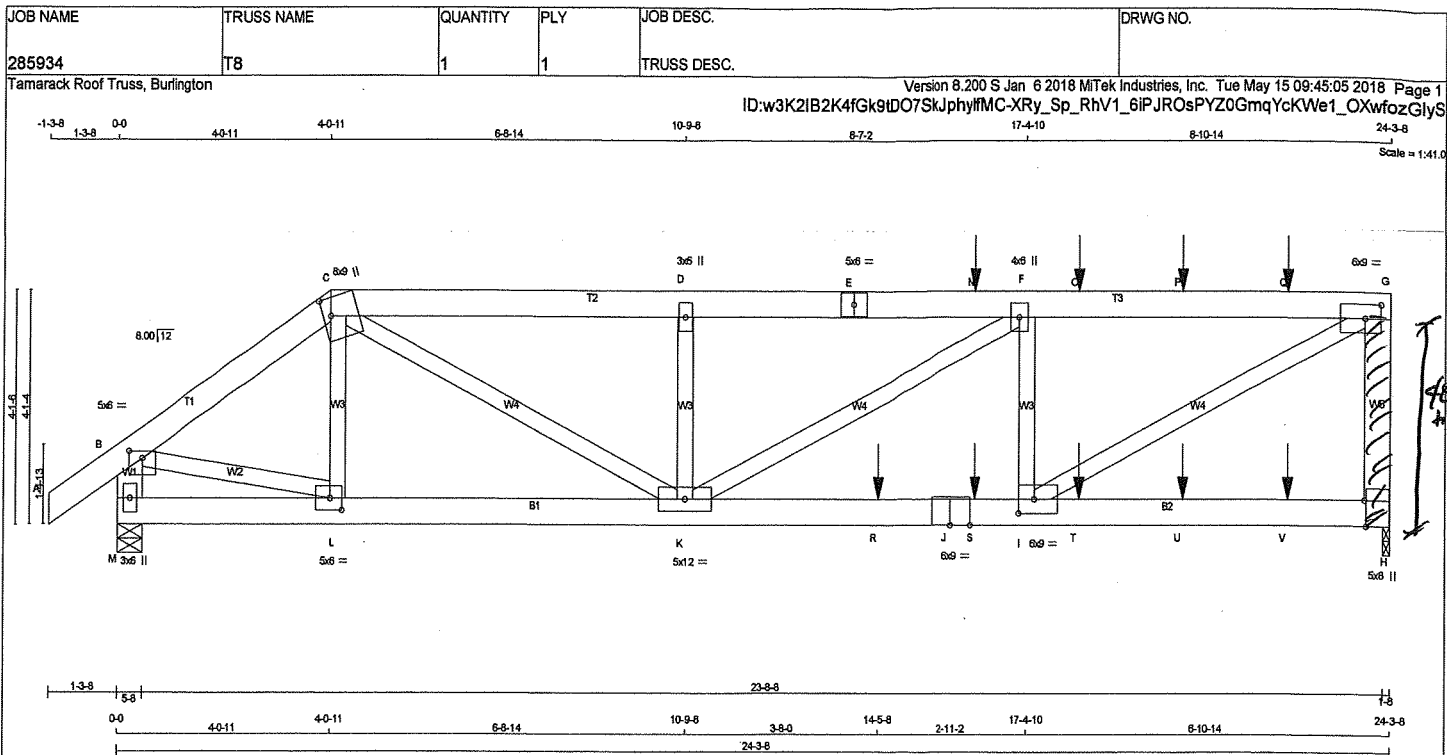
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.93 (M) (INPUT = 1.00)



DRWG NO. TAM 3987-12
STRUCTURAL
COMPONENT ONLY



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x6	DRY No.2	SPF
C - E	2x6	DRY No.2	SPF
E - G	2x6	DRY No.2	SPF
H - G	2x6	DRY No.2	SPF
M - B	2x6	DRY No.2	SPF
M - J	2x6	DRY 2100F 1.8E	SPF
J - H	2x6	DRY 2100F 1.8E	SPF
ALL WEBS EXCEPT	2x4	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TTVW+m	MT20	8.0	9.0	3.75	1.75
D	TMVW+w	MT20	3.0	6.0		
E	TS-t	MT20	5.0	6.0		
F	TMVW+t	MT20	4.0	6.0		
G	TMVW-t	MT20	6.0	9.0	3.00	3.50
H	BMV1+p	MT20	5.0	8.0	Edge	0.25
I	BMVW-t	MT20	6.0	9.0	3.00	3.50
J	BS-t	MT20	6.0	9.0		
K	BMVW-t	MT20	5.0	12.0		
L	BMVW-t	MT20	5.0	6.0	2.50	2.75
M	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) 48" VERTICAL SCAB
SCAB JOINT # 11 WITH 2 X 4#2 SPF 2x4 LONG 1 SIDE(S) USING 2 ROW(S) 2 1/2" ARDOX SPIRAL COMMON NAILS AT 3" O/C STAGGERED (20 NAILS/SCAB) REQUIRED FOR 1 1/2" BRG PROVISION.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT VERT 2719	DOWN 2719	0	0
M VERT 2212	DOWN 2212	0	0

UNFACTORED REACTIONS

1ST CASE	MAX/MIN COMPONENT REACTIONS
JT COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL
H 2211	1262 / 0 480 / 0 0 / 0 0 / 0 488 / 0 0 / 0
M 1781	1046 / 0 367 / 0 0 / 0 0 / 0 368 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.50 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)
FR-TO				FR-TO			
A-B	0 / 33	-84.3	-84.3 0.07 (1)	10.00	L-C	-437 / 17	0.07 (1)
B-C	-2504 / 0	-84.3	-84.3 0.17 (1)	5.05	B-L	0 / 2124	0.38 (1)
C-D	-4685 / 0	-84.3	-84.3 0.45 (1)	3.64	I-G	0 / 5007	0.89 (1)
D-E	-4685 / 0	-84.3	-84.3 0.55 (1)	3.50	C-K	0 / 2958	0.52 (1)
E-N	-4685 / 0	-84.3	-84.3 0.55 (1)	3.50	I-F	-1063 / 0	0.18 (1)
N-F	-4685 / 0	-84.3	-84.3 0.55 (1)	3.50	K-D	-564 / 0	0.10 (1)
F-O	-4428 / 0	-84.3	-84.3 0.61 (1)	3.50	K-F	0 / 272	0.05 (1)
O-P	-4428 / 0	-84.3	-84.3 0.61 (1)	3.50			
P-Q	-4428 / 0	-84.3	-84.3 0.61 (1)	3.50			
Q-G	-4428 / 0	-84.3	-84.3 0.61 (1)	3.50			
H-G	-2691 / 0	0.0	0.0 0.39 (1)	6.32			
M-B	-2131 / 0	0.0	0.0 0.15 (1)	6.93			

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TOTAL
N	18-4-12	-69	-69		BACK	VERT	TOTAL
O	18-4-12	-69	-69		BACK	VERT	TOTAL
P	20-4-12	-69	-69		BACK	VERT	TOTAL
Q	22-4-12	-69	-69		BACK	VERT	TOTAL
R	14-5-8	-1197	-1197		BACK	VERT	TOTAL
S	18-4-12	-50	-50		BACK	VERT	TOTAL
T	18-4-12	-50	-50		BACK	VERT	TOTAL
U	20-4-12	-50	-50		BACK	VERT	TOTAL
V	22-4-12	-50	-50		BACK	VERT	TOTAL

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TOTAL
N	18-4-12	-69	-69		BACK	VERT	TOTAL
O	18-4-12	-69	-69		BACK	VERT	TOTAL
P	20-4-12	-69	-69		BACK	VERT	TOTAL
Q	22-4-12	-69	-69		BACK	VERT	TOTAL
R	14-5-8	-1197	-1197		BACK	VERT	TOTAL
S	18-4-12	-50	-50		BACK	VERT	TOTAL
T	18-4-12	-50	-50		BACK	VERT	TOTAL
U	20-4-12	-50	-50		BACK	VERT	TOTAL
V	22-4-12	-50	-50		BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/968 (0.30")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/615 (0.47")

CSI: TC=0.61/1.00 (F-G:1), BC=0.98/1.00 (I-K:1), WB=0.89/1.00 (G-I:1), SS=0.68/1.00 (I-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.90 (L) (INPUT = 0.90)
JSI METAL= 0.91 (J) (INPUT = 1.00)

CONTINUED ON PAGE 2

DWG NO. TAM 21/54-18 R1
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285934	T8	1	1	TRUSS DESC.	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 101.4 lbs FACTORED DOWN AT
16-4-12, 101.4 lbs FACTORED DOWN AT
18-4-12, AND 101.4 lbs FACTORED DOWN AT
20-4-12, AND 101.4 lbs FACTORED DOWN AT
22-4-12 ON TOP CHORD, AND 1519.0 lbs
FACTORED DOWN AT 14-5-8, 69.9 lbs
FACTORED DOWN AT 16-4-12, 69.9 lbs
FACTORED DOWN AT 18-4-12, AND 69.9 lbs
FACTORED DOWN AT 20-4-12, AND 69.9 lbs
FACTORED DOWN AT 22-4-12 ON BOTTOM
CHORD. DESIGN FOR UNSPECIFIED
CONNECTION(S) IS DELEGATED TO THE
BUILDING DESIGNER.



for

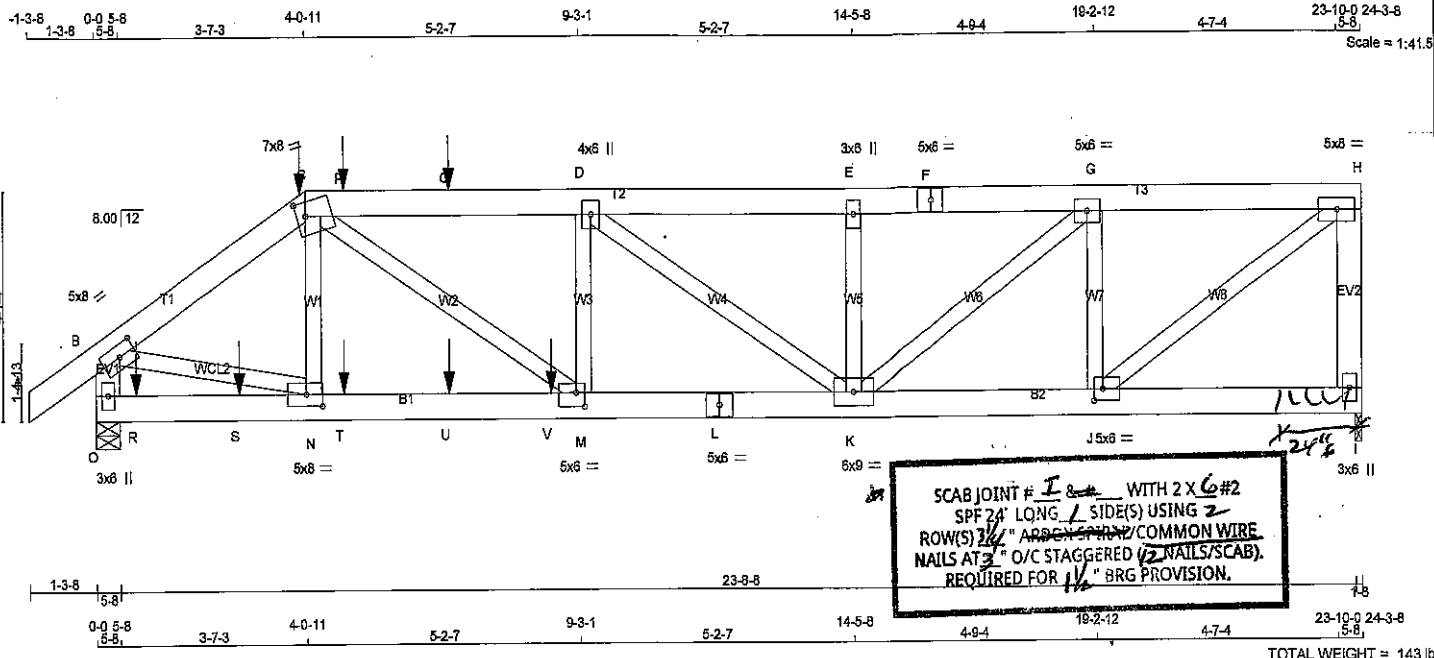
DWG NO. TAM 21154 -18 R1
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285301	T8Z	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 143 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF	
C - F	2x6	DRY	No.2	SPF	
F - H	2x6	DRY	No.2	SPF	
I - H	2x6	DRY	No.2	SPF	
O - B	2x6	DRY	No.2	SPF	
O - L	2x6	DRY	No.2	SPF	
L - I	2x6	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REORD BRG
JT	VERT	HORZ	DOWN	HORZ
I	1894	0	1894	0
O	2731	0	2731	0

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1548	871 / 0	345 / 0	0 / 0	0 / 0	332 / 0	0 / 0
O	2224	1265 / 0	458 / 0	0 / 0	0 / 0	473 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.73 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MEMB.	FORCE (LBS)
FR-TO		FROM TO	FR-TO	
A-B	0 / 33	-84.3 -84.3 0.07 (1)	N-C	-170 / 231
B-C	-3245 / 0	-84.3 -84.3 0.19 (1)	C-M	0 / 2150
C-P	-4458 / 0	-84.3 -84.3 0.41 (1)	M-D	0 / 237
P-Q	-4458 / 0	-84.3 -84.3 0.41 (1)	D-K	-1017 / 0
Q-D	-4458 / 0	-84.3 -84.3 0.41 (1)	K-E	-375 / 0
D-E	-3829 / 0	-84.3 -84.3 0.26 (1)	E-G	0 / 1673
E-F	-3829 / 0	-84.3 -84.3 0.22 (1)	J-G	-1462 / 0
F-G	-3829 / 0	-84.3 -84.3 0.22 (1)	J-H	0 / 2633
G-H	-2300 / 0	-84.3 -84.3 0.18 (1)	B-N	0 / 2753
I-H	-1853 / 0	0.0 0.0 0.27 (1)		
O-B	-2875 / 0	0.0 0.0 0.19 (1)		
O-R	0 / 0	-28.0 -28.0 0.19 (2)		
R-S	0 / 0	-28.0 -28.0 0.19 (2)		
S-N	0 / 0	-28.0 -28.0 0.19 (2)		
N-T	0 / 2686	-28.0 -28.0 0.67 (1)		
T-U	0 / 2686	-28.0 -28.0 0.67 (1)		
U-V	0 / 2686	-28.0 -28.0 0.67 (1)		
V-M	0 / 2686	-28.0 -28.0 0.67 (1)		
M-L	0 / 4458	-28.0 -28.0 0.77 (1)		
L-K	0 / 4458	-28.0 -28.0 0.77 (1)		
K-J	0 / 2300	-28.0 -28.0 0.37 (1)		
J-I	0 / 0	-28.0 -28.0 0.08 (3)		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	4-0-11	-23	-25		FRONT	VERT	DEAD
C	4-0-11	-222	-222		FRONT	VERT	SNOW
P	4-9-4	-113	-113		FRONT	VERT	TOTAL
Q	6-8-4	-101	-101		FRONT	VERT	TOTAL
R	9-4	-43	-76		FRONT	VERT	TOTAL
S	2-9-4	-40	-70		FRONT	VERT	TOTAL
T	4-9-4	-40	-70		FRONT	VERT	TOTAL
U	6-9-4	-40	-70		FRONT	VERT	TOTAL
V	8-8-8	-1157	-1157		FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. G.C.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/999 (0.14")
ALLOWABLE DEFL.(TL) = L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/999 (0.22")

CSI: TC=0.41/1.00 (C-D:1), BC=0.77/1.00 (K-M:1), WB=0.50/1.00 (H-J:1), SSI=0.74/1.00 (M-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1887 822 2284 1656

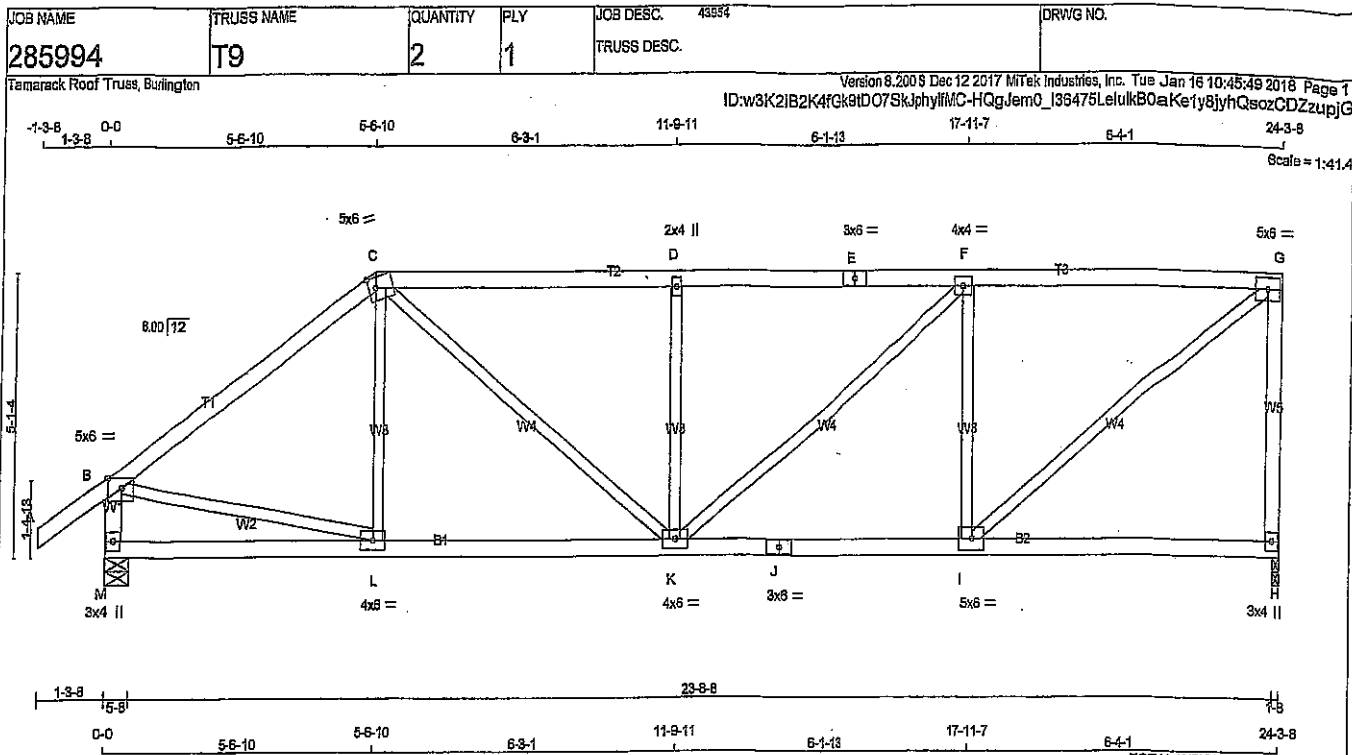
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.90 (C) (INPUT = 0.90)
JSI METAL=0.65 (L) (INPUT = 1.00)

DWG NO. TAM 2663-8
STRUCTURAL
COMPONENT ONLY





LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESGR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
H - G	2x4	DRY	No.2
M - B	2x4	DRY	No.2
M - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
DRY: SEASONED LUMBER.			

PLATES (tablets in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMW-p	MT20	5.0	6.0	Edge
C	TTWW-m	MT20	5.0	6.0	2.25 1.75
D	TMW-w	MT20	2.0	4.0	
E	TS-t	MT20	3.0	6.0	
F	TMW-w	MT20	4.0	4.0	
G	TMW-w	MT20	5.0	6.0	
H	BMV1-p	MT20	3.0	4.0	
I	BMW-w	MT20	5.0	6.0	
J	BS-t	MT20	3.0	6.0	
K	BMW-w	MT20	4.0	6.0	
L	BMW-w	MT20	4.0	6.0	
M	BMV1-p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT VERT	DOWN	0	0
H 1363	0	1-8	1-8
M 1479	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS
JT COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL
H 1119	821/0 255/0 0/0 0/0 243/0 0/0
M 1198	892/0 255/0 0/0 0/0 251/0 0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.34 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)
FR-TO				FR-TO			
A-B	0/32	-84.3	-84.3 0.11 (1)	L-C	-49/204	0.05 (3)	0.05 (3)
B-C	-1498/0	-84.3	-84.3 0.55 (1)	B-L	0/1263	0.28 (1)	0.28 (1)
C-D	-1797/0	-84.3	-84.3 0.55 (1)	I-G	0/1778	0.40 (1)	0.40 (1)
D-E	-1797/0	-84.3	-84.3 0.55 (1)	C-K	0/1702	0.16 (1)	0.16 (1)
E-F	-1797/0	-84.3	-84.3 0.55 (1)	I-F	-877/0	0.34 (1)	0.34 (1)
F-G	-1415/0	-84.3	-84.3 0.53 (1)	K-D	-566/0	0.22 (1)	0.22 (1)
H-G	-1292/0	0.0	0.0 0.57 (1)	K-F	0/484	0.11 (1)	0.11 (1)
M-B	-1417/0	0.0	0.0 0.15 (1)				
M-L	0/0	-28.0	-28.0 0.24 (3)				
L-K	0/1240	-28.0	-28.0 0.39 (2)				
K-J	0/1416	-28.0	-28.0 0.44 (2)				
J-I	0/1416	-28.0	-28.0 0.44 (2)				
I-H	0/0	-28.0	-28.0 0.27 (3)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 3.0 PSF
BOT CH. LL	= 10.5 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 46.1 PSF

SPACING = 24.0 IN. GIC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/999 (0.09")
ALLOWABLE DEFL.(TL) = L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/999 (0.14")

CSI: TC=0.57/1.00 (G-H-1), BC=0.44/1.00 (I-K-2), WB=0.40/1.00 (G-I-1), SS=0.25/1.00 (F-G-1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

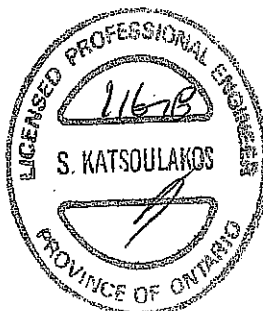
NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

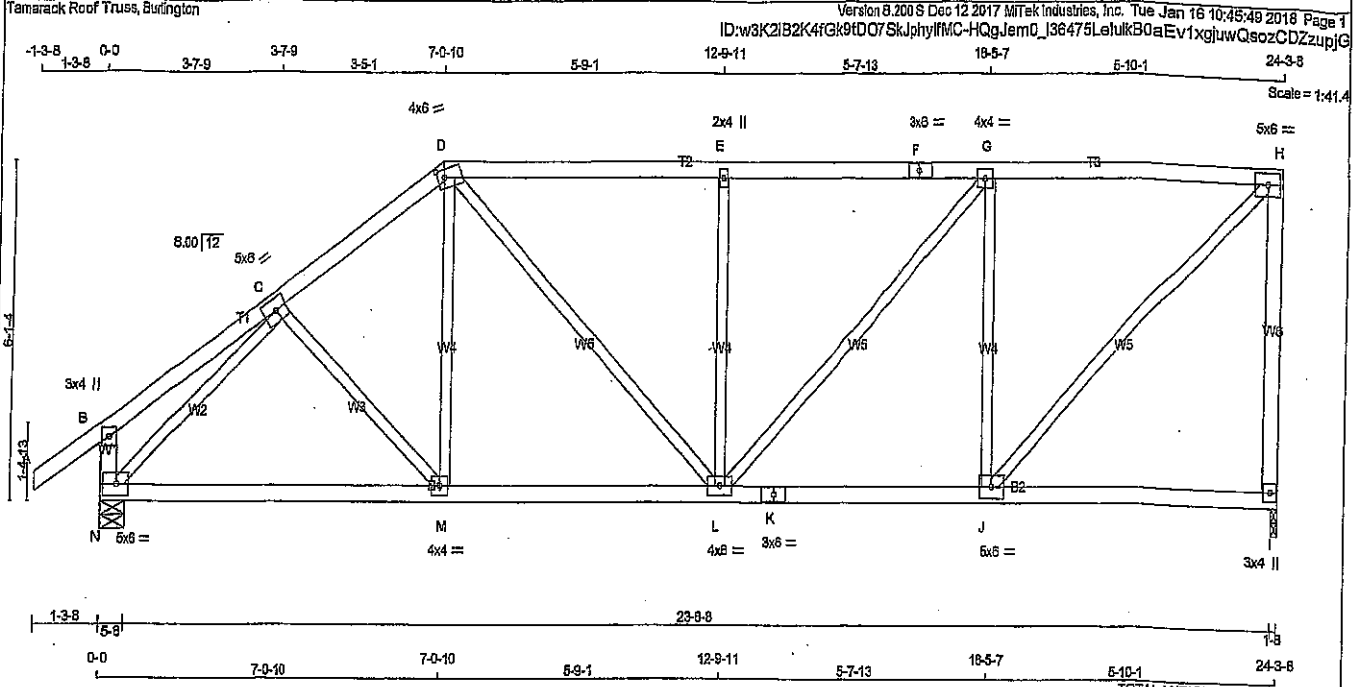
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (K) (INPUT = 0.90)
JSI METAL= 0.44 (J) (INPUT = 1.00)



DWG NO. TAM 3900 -18
STRUCTURAL
COMPONENT ONLY

JOB NAME 285994	TRUSS NAME T10	QUANTITY 2	PLY 1	JOB DESC. 43934	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.200 S Dec 12 2017 Mitek Industries, Inc. Tue Jan 16 10:45:49 2018 Page 1 ID: w3K2iB2K4fGk9tD07SkJphYfMC-HQgJem0_J364751etukB0aEv1xglwQsozCDZupjG	



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW-m	MT20	4.0	6.0	1.75	1.75
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TMWW-t	MT20	5.0	6.0		
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	5.0	6.0		
K	BS-t	MT20	3.0	6.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMWW-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	HORIZ	UPLIFT
I	1393	0	1393	0
N	1479	0	1479	0

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS
JT	COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL
I	1119 621/0 255/0 0/0 0/0 243/0 0/0
N	1198 692/0 255/0 0/0 0/0 251/0 0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.86 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1 MAX)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC1 MAX)	MAX. FACTORED VERT. LOAD (LC1 MAX)
FR-TO				FR-TO			
A-B	0/32	-84.3	-84.3 0.11 (1)	10.00	C-M	0/116	0.03 (3)
B-C	0/17	-84.3	-84.3 0.15 (1)	10.00	M-D	0/307	0.07 (2)
C-O	-1460/0	-84.3	-84.3 0.18 (1)	5.22	N-C	-1699/0	0.04 (1)
D-E	-1473/0	-84.3	-84.3 0.43 (1)	4.86	J-H	0/1557	0.35 (1)
E-F	-1473/0	-84.3	-84.3 0.44 (1)	4.86	D-L	0/365	0.08 (1)
F-G	-1473/0	-84.3	-84.3 0.44 (1)	4.86	J-G	-912/0	0.53 (1)
G-H	-1103/0	-84.3	-84.3 0.42 (1)	5.45	L-E	-520/0	0.30 (1)
I-H	-1298/0	0.0	0.0 0.94 (1)	7.06	L-G	0/830	0.12 (1)
N-B	-233/0	0.0	0.0 0.02 (1)	7.81			
N-M	0/1196	-28.0	-28.0 0.45 (2)	10.00			
M-L	0/1202	-28.0	-28.0 0.47 (2)	10.00			
L-K	0/1103	-28.0	-28.0 0.37 (2)	10.00			
K-J	0/1103	-28.0	-28.0 0.37 (2)	10.00			
J-I	0/0	-28.0	-28.0 0.24 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 3.0 PSF

BOT CH. LL = 10.5 PSF
DL = 7.0 PSF

TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2012, BCBC 2012, ABC 2014
- CSA 088-08
- TPIC 2011

(85 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL) = L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/999 (0.19")

CS: TC=0.94/1.00 (H-H), BC=0.47/1.00 (L-M-2),
WB=0.64/1.00 (C-N-1), SS=0.23/1.00 (G-H-1)

DOL LUMBER=1.00 NAL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1687
	822	2284	1686

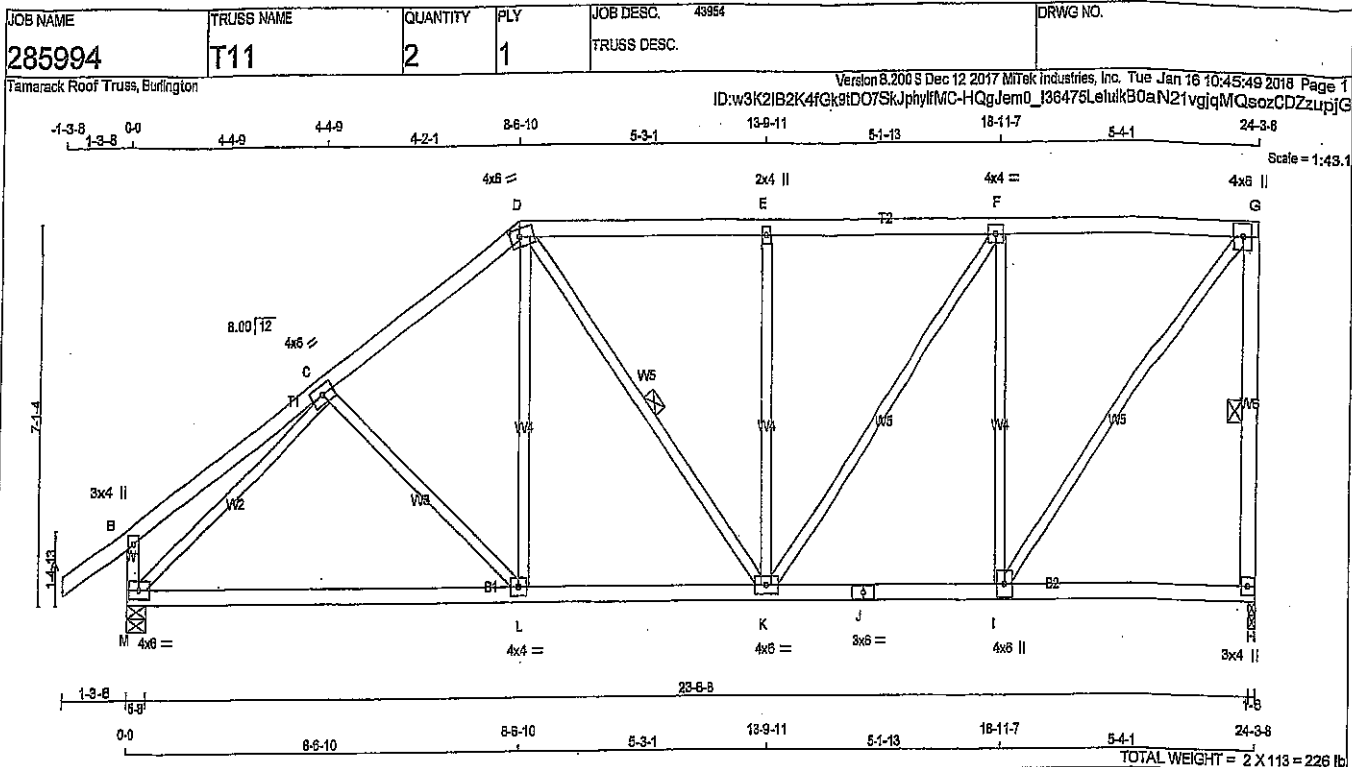
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.80 (D) (INPUT = 0.90)
JSI METAL= 0.41 (C) (INPUT = 1.00)



DRWG NO. TAM 3989-18
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
 EXCEPT
 DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW+t	MT20	4.0	6.0		
D	TTVW+m	MT20	4.0	6.0	1.75	2.50
E	TMVW+w	MT20	2.0	4.0		
F	TMVW+t	MT20	4.0	4.0		
G	TMVW+p	MT20	4.0	6.0		
H	BMV+p	MT20	3.0	4.0		
I	BMVW+t	MT20	4.0	6.0		
J	BS+t	MT20	3.0	6.0		
K	BMVWV+t	MT20	4.0	6.0		
L	BMVW+t	MT20	4.0	4.0		
M	BMVWV+t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS:

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	1-8	1-8
M	HORIZ	UPLIFT	5-8	5-8

UNFACTORED REACTIONS

	1ST CASE	MAX/MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL
H	1119	621/0 255/0 0/0 0/0 243/0 0/0
M	1166	692/0 255/0 0/0 0/0 251/0 0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.20 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-H, D-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
 TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	
FR-TO		FROM TO	CS1 (LX)	FR-TO			
A-B	0/32	-84.3 -84.3	0.11 (1)	C-L	-117/85	0.07 (1)	
B-C	0/23	-84.3 -84.3	0.24 (1)	L-D	0/414	0.08 (2)	
C-D	-1360/0	-84.3 -84.3	0.28 (1)	M-C	-1661/0	0.93 (1)	
D-E	-1226/0	-84.3 -84.3	0.34 (1)	I-G	0/1426	0.32 (1)	
E-F	-1227/0	-84.3 -84.3	0.35 (1)	D-K	-8/141	0.03 (1)	
F-G	-879/0	-84.3 -84.3	0.34 (1)	I-F	-948/0	0.83 (1)	
H-G	-1305/0	0.0 0.0	0.30 (1)	K-E	-474/0	0.41 (1)	
M-B	-255/0	0.0 0.0	0.03 (1)	K-F	0/576	0.13 (1)	
M-L	0/1223	-28.0 -28.0	0.58 (2)				
L-K	0/1141	-28.0 -28.0	0.59 (2)				
K-J	0/879	-28.0 -28.0	0.31 (2)				
J-I	0/879	-28.0 -28.0	0.31 (2)				
I-H	0/0	-28.0 -28.0	0.21 (3)				

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 25.6 PSF
 DL = 3.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF CBC 2012, BCBC 2012, ABC 2014
 - CSA 085-09
 - TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.81")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.23")
 ALLOWABLE DEFL.(TL) = L/360 (0.81")
 CALCULATED VERT. DEFL.(TL) = L/779 (0.37")

CSI: TC=0.35/1.00 (E-F-1), BC=0.59/1.00 (K-L-2),
 WS=0.93/1.00 (C-M-1), SS=0.21/1.00 (F-G-1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

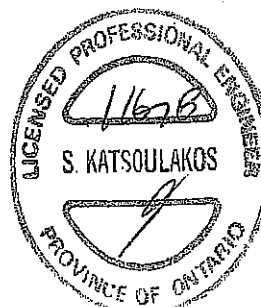
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION
 (PSI) (PL) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1687 822 2284 1656

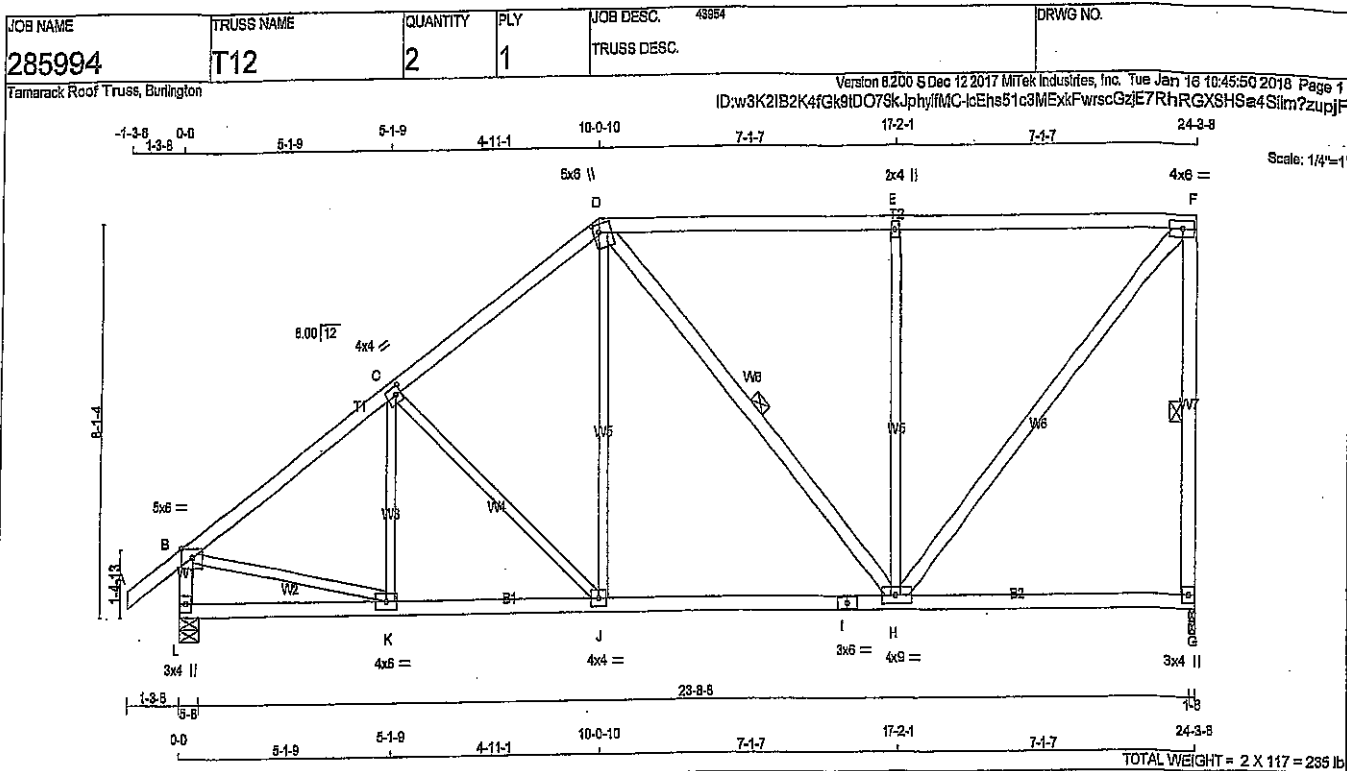
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (M) (INPUT = 0.90)
 JSI METAL= 0.40 (C) (INPUT = 1.00)



DRWG NO. TAM 3990-18
 STRUCTURAL
 COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
G - F	2x4	DRY	No.2
L - B	2x4	DRY	No.2
L - I	2x4	DRY	No.2
I - G	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
D - H	2x4	DRY	No.2
H - F	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge
C	TMVW-i	MT20	4.0	4.0	2.00 1.50
D	TMVW+m	MT20	5.0	6.0	2.00 1.50
E	TMVW-w	MT20	2.0	4.0	
F	TMVW-i	MT20	4.0	6.0	
G	BMV1+p	MT20	3.0	4.0	
H	BMVWV-i	MT20	4.0	9.0	
I	BS-t	MT20	3.0	6.0	
J	BMVW-i	MT20	4.0	4.0	
K	BMVW-i	MT20	4.0	6.0	
L	BMV1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
	VERT HORZ	DOWN HORZ	UPLIFT IN-SX	IN-SX
JT	1363 0	1363 0	0 1-8	1-8
G	1479 0	1479 0	0 5-8	5-8
L				

UNFACTORED REACTIONS

	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	1119	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
G	1198	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0
L							

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.81 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX. (LC)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED (LC)
FR-TO					FR-TO		
A-B	0 / 32	-84.3	-84.3 0.11 (1)	10.00	K-C	-123 / 133	0.04 (1)
B-C	-1528 / 0	-84.3	-84.3 0.43 (1)	4.83	C-J	-347 / 0	0.29 (1)
C-D	-1275 / 0	-84.3	-84.3 0.40 (1)	5.20	J-D	0 / 438	0.10 (2)
D-E	-987 / 0	-84.3	-84.3 0.80 (1)	4.81	D-H	-110 / 0	0.08 (1)
E-F	-967 / 0	-84.3	-84.3 0.61 (1)	4.81	H-E	-745 / 0	0.94 (1)
G-F	-1284 / 0	0.0	0.0 0.38 (1)	5.89	H-F	0 / 1432	0.23 (1)
L-B	-1418 / 0	0.0	0.0 0.15 (1)	6.85	B-K	0 / 1323	0.30 (1)
L-K	0 / 0	-28.0	-28.0 0.16 (3)	10.00			
K-J	0 / 1295	-28.0	-28.0 0.33 (2)	10.00			
J-I	0 / 1041	-28.0	-28.0 0.49 (2)	10.00			
I-H	0 / 1041	-28.0	-28.0 0.49 (2)	10.00			
H-G	0 / 0	-28.0	-28.0 0.37 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, BOBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(65% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.81")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.10")
ALLOWABLE DEFL.(TL) = $L/360$ (0.81")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.15")

CSI: TC=0.61/1.00 (E-F:1), BC=0.49/1.00 (H-J:2),
WB=0.94/1.00 (E-H:1), SS=0.29/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1657 822 2284 1555

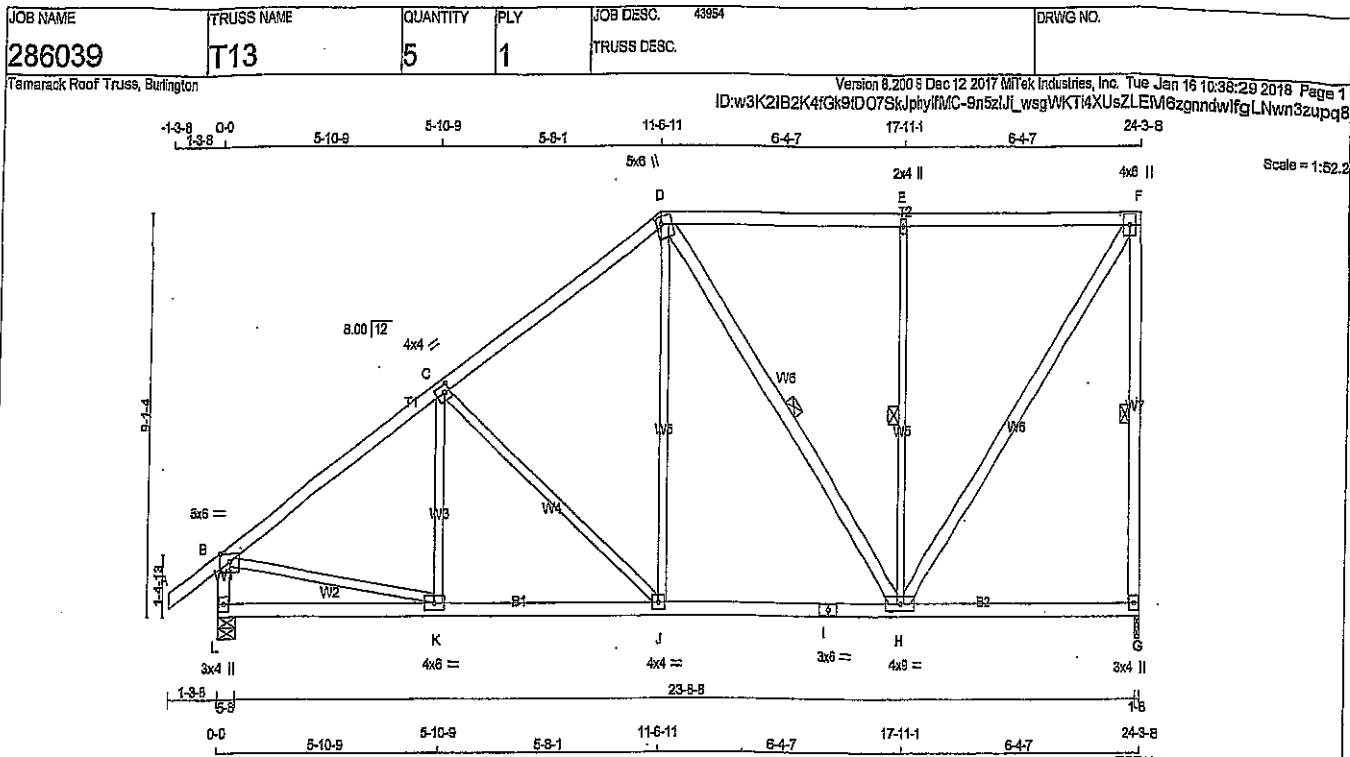
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.80)
JSI METAL= 0.36 (I) (INPUT = 1.00)



DRWG NO. TAM 399.1 -18
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
G - F	2x4	DRY	No.2
L - B	2x4	DRY	No.2
L - I	2x4	DRY	No.2
I - G	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
D - H	2x4	DRY	No.2
H - F	2x4	DRY	No.2

SPF

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMW-p	MT20	6.0	6.0	Edge
C	TMW-w	MT20	4.0	4.0	2.00 1.50
D	TTW-w	MT20	5.0	6.0	2.00 1.50
E	TMW-w	MT20	2.0	4.0	
F	TMW-w	MT20	4.0	6.0	
G	BMW-w	MT20	3.0	4.0	
H	BMW-w	MT20	4.0	6.0	
I	BS-w	MT20	3.0	6.0	
J	BMW-w	MT20	4.0	4.0	
K	BMW-w	MT20	4.0	6.0	
L	BMW-w	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECORD BRG
	VERT	HORZ	DOWN	HORZ		
G	1353	0	1353	0	1-8	1-8
L	1479	0	1479	0	5-8	5-8

UNFACTORED REACTIONS

JT	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
G	1119	621/0	255/0	0/0	0/0	243/0
L	1198	892/0	255/0	0/0	0/0	251/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. FURLIN SPACING = 4.81 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF FG, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0/32	-84.3 -84.3 0.11 (1)	10.00	K-C	-58/203	0.05 (3)	
B-C	-1531/0	-84.3 -84.3 0.59 (1)	4.61	C-J	-474/0	0.58 (1)	
C-D	-1187/0	-84.3 -84.3 0.54 (1)	5.18	J-D	0/490	0.11 (2)	
D-E	-790/0	-84.3 -84.3 0.61 (1)	5.73	D-H	-270/0	0.17 (1)	
E-F	-790/0	-84.3 -84.3 0.62 (1)	5.73	H-E	-686/0	0.38 (1)	
G-F	-1282/0	0.0 0.0 0.50 (1)	5.68	H-F	0/1344	0.22 (1)	
L-B	-1412/0	0.0 0.0 0.15 (1)	6.85	B-K	0/1322	0.30 (1)	
L-K	0/0	-28.0 -28.0 0.23 (3)	10.00				
K-J	0/1300	-28.0 -28.0 0.38 (2)	10.00				
J-I	0/848	-28.0 -28.0 0.40 (2)	10.00				
I-H	0/846	-28.0 -28.0 0.40 (2)	10.00				
H-G	0/0	-28.0 -28.0 0.29 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC0 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA C88-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CSI: TC=0.62/1.00 (E-F:1), BC=0.40/1.00 (H-J:2), WB=0.58/1.00 (C-J:1), SB=0.26/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 616 354 1667 822 2284 1655

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.90 (H) (INPUT = 0.90)
JSI METAL=0.32 (K) (INPUT = 1.00)



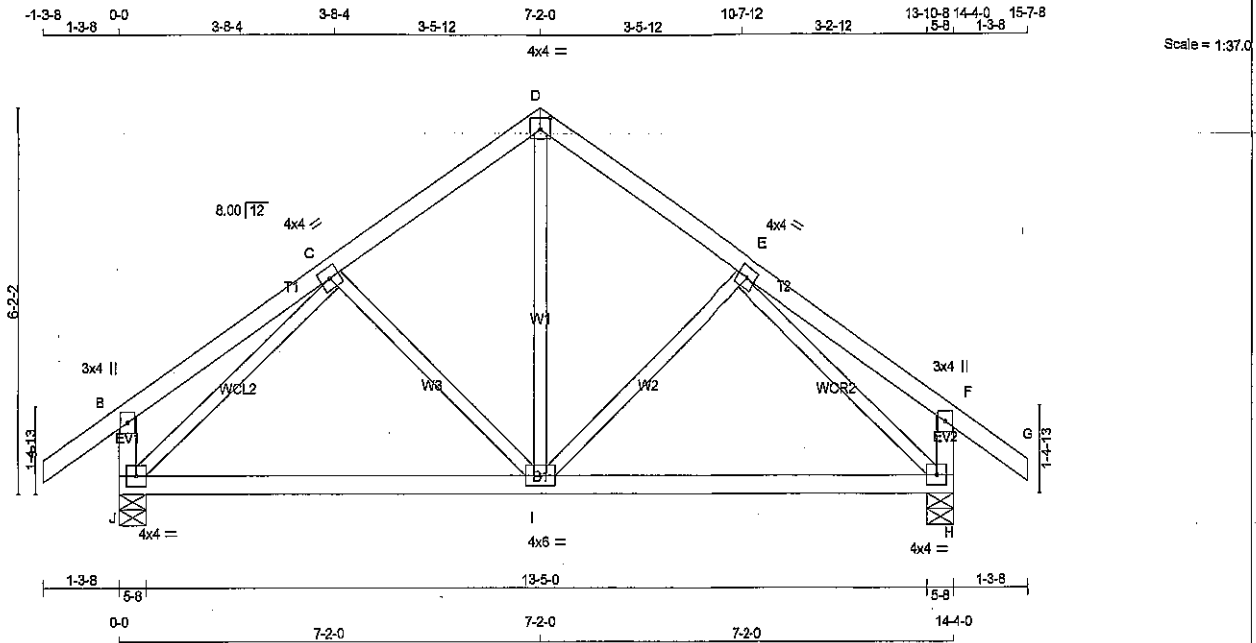
DWG NO. TAM 3993-12
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
285781	T14	12	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:Uer6HFwTUsfudnkBu50lkHyEuR-zlPaUNU36va_zWSKhGc_qTeNa7wn25tyNZD1azP997



TOTAL WEIGHT = 12 X 62 = 741 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
J - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	3.0	4.0
C	TMWw-t	MT20	4.0	4.0
D	TTW-p	MT20	4.0	4.0
E	TMWw-t	MT20	4.0	4.0
F	TMV+p	MT20	3.0	4.0
H	BMVW1-t	MT20	4.0	4.0
I	BMWwW1-t	MT20	4.0	6.0
J	BMVW1-t	MT20	4.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	HORZ	UPLIFT	IN-SX
J	920	0	920	0	5-8
H	920	0	920	0	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS
JT	COMBINED
J	739
H	739

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED			
MEMB.	FORCE	VERT. LOAD	LC1 MAX	MAX.	MEMB.	FORCE	MAX
	(LBS)	(PLF)	CSI (LC)	UNBRAC		(LBS)	CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
A-B	0/32	-84.3	-84.3 0.11 (1)	10.00	I-D	0/460	0.10 (1)
B-C	0/19	-84.3	-84.3 0.17 (1)	10.00	I-E	-152/44	0.08 (1)
C-D	-845/0	-84.3	-84.3 0.13 (1)	6.25	C-I	-152/44	0.06 (1)
D-E	-845/0	-84.3	-84.3 0.13 (1)	6.25	J-C	-879/0	0.35 (1)
E-F	0/19	-84.3	-84.3 0.17 (1)	10.00	E-H	-879/0	0.35 (1)
F-G	0/32	-84.3	-84.3 0.11 (1)	10.00			
J-B	-232/0	0.0	0.0 0.02 (1)	7.81			
H-F	-232/0	0.0	0.0 0.02 (1)	7.81			
J-I	0/631	-28.0	-28.0 0.49 (2)	10.00			
I-H	0/631	-28.0	-28.0 0.49 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.48")
CALCULATED VERT. DEFL.(LL)= L/999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.48")
CALCULATED VERT. DEFL.(TL)= L/999 (0.13")

CSI: TC=0.17/1.00 (E-F:1), BC=0.49/1.00 (H-I:2), WB=0.35/1.00 (E-H:1), SS=0.17/1.00 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (D) (INPUT = 0.90)
JSI METAL= 0.31 (C) (INPUT = 1.00)



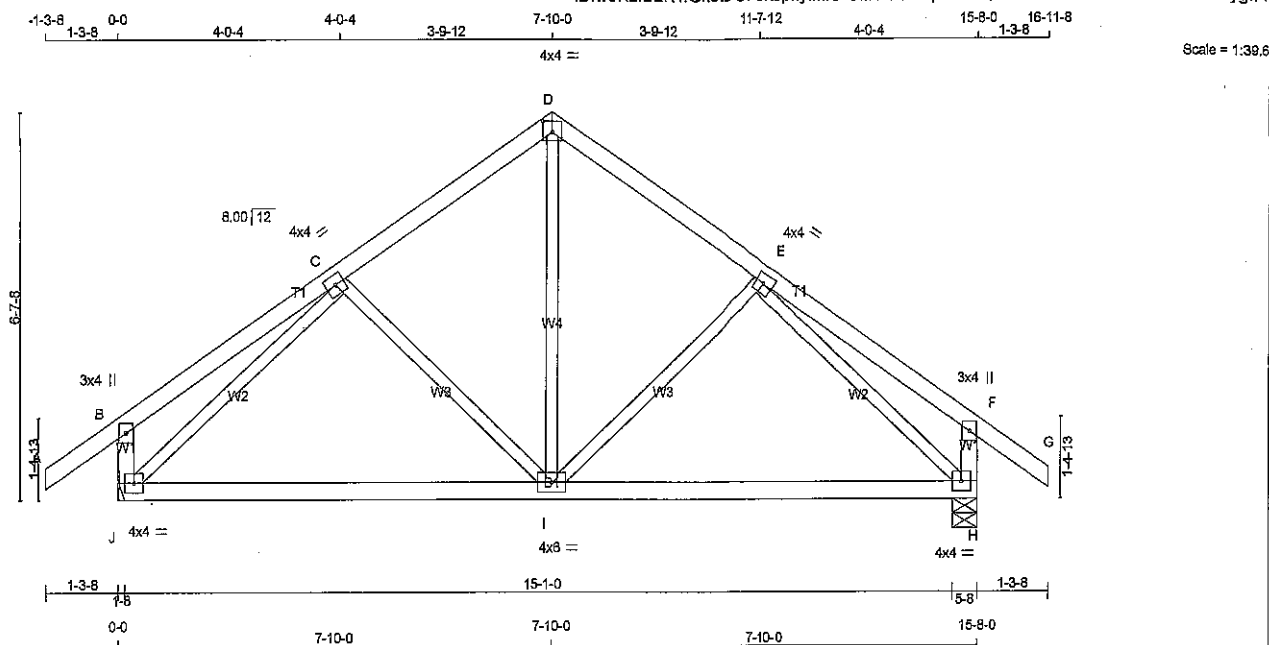
DRWG NO. TAM 21200-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
285781	T15	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:w3K2IB2K4fGk9DO7SkJphylfWC-ULN0BM5kpJe9PQ28HFnDsdzVb15MTwBMFTNZDygfFr



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
J - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BMWW-t	MT20	4.0	4.0		
I	BMWW-t	MT20	4.0	6.0		
J	BMWW-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
J	995	0	995	0	0	
H	995	0	995	0	0	

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
J	800	471 / 0	165 / 0	0 / 0	0 / 0	165 / 0	0 / 0
H	800	471 / 0	165 / 0	0 / 0	0 / 0	165 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED			
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX. (LC)	MAX. UNBRACED LENGTH	MEMB.	FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 32	-84.3	-84.3 0.11 (1)	10.00	I-D	0 / 516	0.12 (1)
B-C	0 / 21	-84.3	-84.3 0.20 (1)	10.00	I-E	-179 / 43	0.09 (1)
C-D	-717 / 0	-84.3	-84.3 0.16 (1)	6.25	C-I	-179 / 43	0.09 (1)
D-E	-717 / 0	-84.3	-84.3 0.16 (1)	6.25	J-C	-975 / 0	0.46 (1)
E-F	0 / 21	-84.3	-84.3 0.20 (1)	10.00	E-H	-975 / 0	0.46 (1)
F-G	0 / 32	-84.3	-84.3 0.11 (1)	10.00			
J-B	-243 / 0	0.0	0.0 0.02 (1)	7.81			
H-F	-243 / 0	0.0	0.0 0.02 (1)	7.81			
J-I	0 / 709	-28.0	-28.0 0.58 (2)	10.00			
I-H	0 / 709	-28.0	-28.0 0.58 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.52")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL) = L/360 (0.52")
CALCULATED VERT. DEFL.(TL) = L/999 (0.18")

CSI: TO=0.20 (B-C:1), BC=0.58 (H-I:2), WB=0.48 (C-J:1), SSI=0.19 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1697 622 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (D) (INPUT = 0.90)
JSI METAL= 0.35 (C) (INPUT = 1.00)



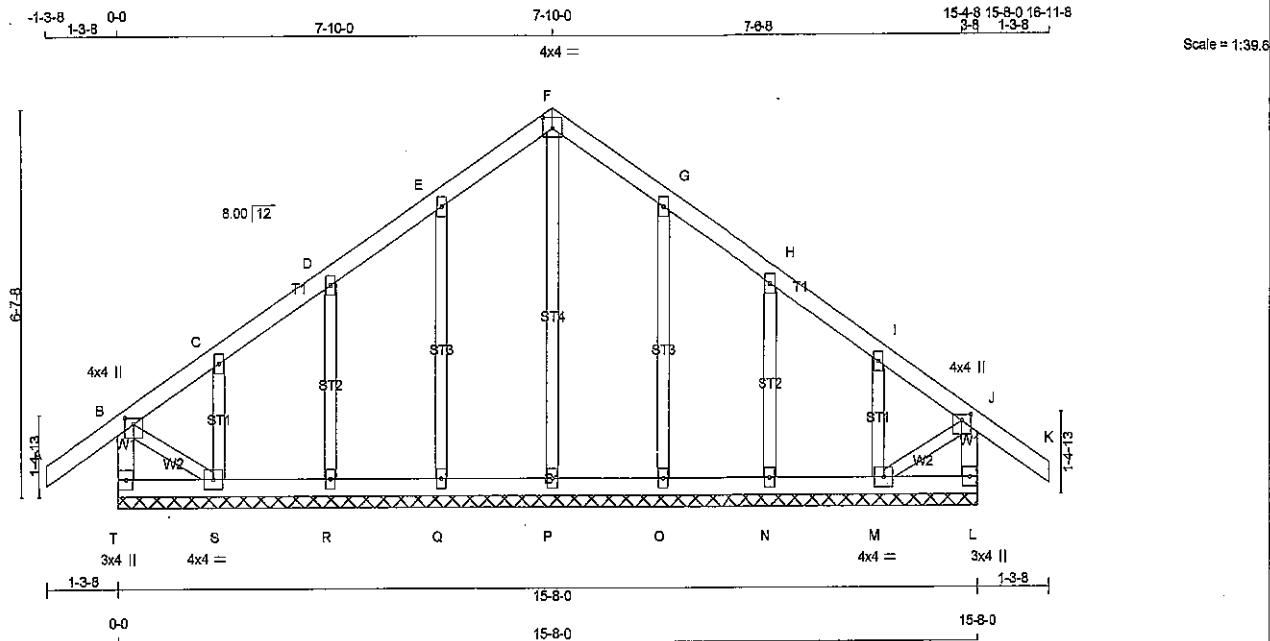
DWG NO. TAM 445717
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43994	DRWG NO.
285781	G15	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:w3K2IB2K4fGk9tDO7SkJphylfMC-B?SMjy0LT9m83L0oNH9a48BLmxrsDPu9I3GWp7ygfFy



TOTAL WEIGHT = 70 lb

LUMBER

N. L. G. A. RULES					
CHORDS		SIZE	LUMBER		DESCR
T - B	2x4	DRY	No.2		SPF
A - F	2x4	DRY	No.2		SPF
F - K	2x4	DRY	No.2		SPF
L - J	2x4	DRY	No.2		SPF
T - L	2x4	DRY	No.2		SPF

ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2'-0" O.C.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C, D, E, G, H, I						
C	TMVW+w	MT20	2.0	4.0		
F	TTW+p	MT20	4.0	4.0	2.25	2.00
J	TMVW+p	MT20	4.0	4.0	1.25	2.00
L	BMV1+p	MT20	3.0	4.0		
M	BMVW1-t	MT20	4.0	4.0		
N, O, P, Q, R						
N	BMV1+w	MT20	2.0	4.0		
S	BMVW1-t	MT20	4.0	4.0		
T	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	LC1 MAX (LC)
FR-TO		FROM TO		FR-TO			
T-B	-253 / 0	0.0	0.0 0.03 (1)	P-F	-129 / 0	7.81	0.09 (1)
A-B	0 / 32	-84.3	-84.3 0.11 (1)	Q-E	-189 / 0	10.00	0.08 (1)
B-C	-48 / 0	-84.3	-84.3 0.11 (1)	R-D	-172 / 0	6.25	0.04 (1)
C-D	-15 / 0	-84.3	-84.3 0.04 (1)	S-C	-108 / 0	6.25	0.02 (1)
D-E	-14 / 0	-84.3	-84.3 0.05 (1)	C-G	-189 / 0	6.25	0.08 (1)
E-F	-21 / 0	-84.3	-84.3 0.05 (1)	N-H	-172 / 0	6.25	0.04 (1)
F-G	-21 / 0	-84.3	-84.3 0.05 (1)	M-I	-108 / 0	6.25	0.02 (1)
G-H	-14 / 0	-84.3	-84.3 0.05 (1)	B-S	0 / 23	6.25	0.01 (1)
H-I	-15 / 0	-84.3	-84.3 0.04 (1)	M-J	0 / 23	6.25	0.01 (1)
I-J	-48 / 0	-84.3	-84.3 0.11 (1)				
J-K	0 / 32	-84.3	-84.3 0.11 (1)				
L-J	-253 / 0	0.0	0.0 0.03 (1)				
T-S	0 / 0	-28.0	-28.0 0.03 (2)				
S-R	0 / 16	-28.0	-28.0 0.03 (2)				
R-Q	0 / 12	-28.0	-28.0 0.02 (2)				
Q-P	0 / 9	-28.0	-28.0 0.02 (2)				
P-O	0 / 9	-28.0	-28.0 0.02 (2)				
O-N	0 / 12	-28.0	-28.0 0.02 (2)				
N-M	0 / 16	-28.0	-28.0 0.03 (2)				
M-L	0 / 0	-28.0	-28.0 0.03 (2)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD	=	46.1	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11 (J-K-1), BC=0.03 (R-S-2), WB=0.09 (F-P-1), SS=0.08 (J-K-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PS)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1867 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

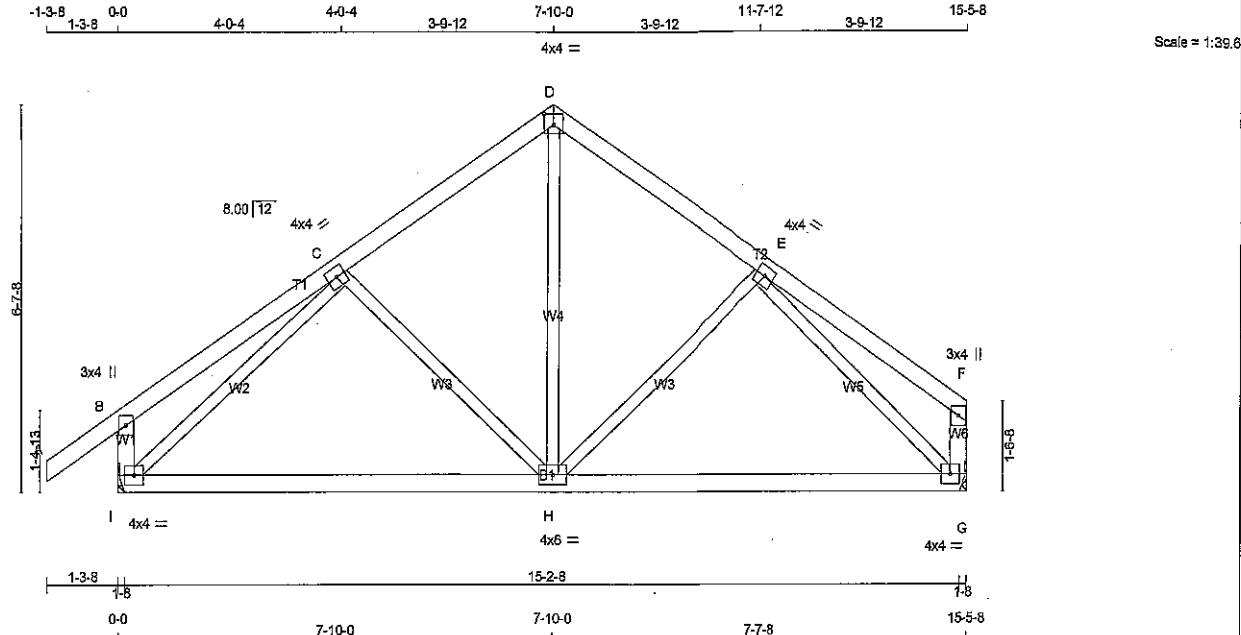
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (B) (INPUT = 0.90)

JSI METAL= 0.05 (G) (INPUT = 1.00)



DWG NO. TAM 4488817
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 2 X 65 = 129 lb
(MIF)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-1	MT20	4.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMVW-1	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-1	MT20	4.0	4.0		
H	BMVW-1	MT20	4.0	6.0		
I	BMVW1-1	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
I	983	0	983	0	0	
G	868	0	868	0	0	

MIN. SEAT SIZE: 1-8
HANGER BY OTHERS

UNFACTORED REACTIONS

JT	COMBINED	MAX./MIN. COMPONENT REACTIONS				
		SNOW	LIVE	PERM.LIVE	WIND	DEAD
I	791	468 / 0	182 / 0	0 / 0	0 / 0	183 / 0
G	712	395 / 0	162 / 0	0 / 0	0 / 0	155 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING									
TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO				FR-TO			
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	H-D	0 / 494	0.11 (1)	
B-C	0 / 21	-84.3	-84.3	0.20 (1)	10.00	H-E	-148 / 53	0.07 (1)	
C-D	-698 / 0	-84.3	-84.3	0.16 (1)	6.25	C-H	-182 / 41	0.08 (1)	
D-E	-697 / 0	-84.3	-84.3	0.15 (1)	6.25	I-C	-957 / 0	0.45 (1)	
E-F	0 / 21	-84.3	-84.3	0.19 (1)	10.00	E-G	-948 / 0	0.42 (1)	
I-B	-242 / 0	0.0	0.0	0.02 (1)	7.81				
G-F	-118 / 0	0.0	0.0	0.01 (1)	7.81				
I-H	0 / 698	-28.0	-28.0	0.56 (2)	10.00				
H-G	0 / 672	-28.0	-28.0	0.56 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.52")
CALCULATED VERT. DEFL.(LL) = L/899 (0.11")
ALLOWABLE DEFL.(TL) = L/360 (0.52")
CALCULATED VERT. DEFL.(TL) = L/981 (0.19")

CSI: TC=0.20 (B-C:1), BC=0.56 (H-I:2), WB=0.45 (C-I:1), SSI=0.19 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

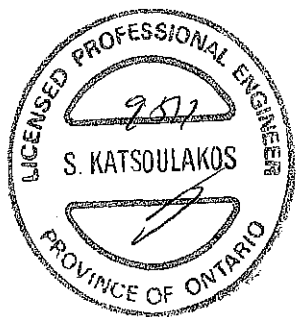
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	818	354	1667
	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (D) (INPUT = 0.90)
JSI METAL= 0.34 (C) (INPUT = 1.00)



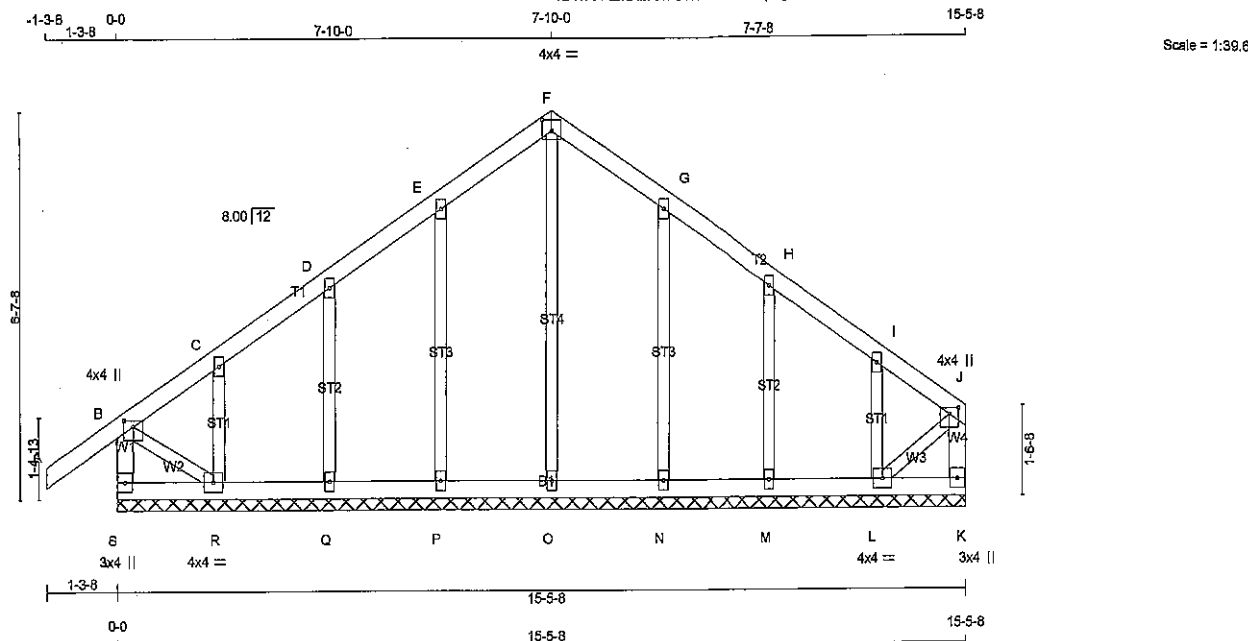
DWG NO. TAM 4485B-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
285781	G15A	1	1	TRUSS DESC.		

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Scale = 1:39.6

TOTAL WEIGHT = 68 lb

LUMBER	CHORDS	SIZE	DRY	No.2	DESCR.
N. L. G. A. RULES					
S - B	2x4	DRY	No.2	SPF	
A - F	2x4	DRY	No.2	SPF	
F - J	2x4	DRY	No.2	SPF	
K - J	2x4	DRY	No.2	SPF	
S - K	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
ALL GABLE WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

GABLE STUDS SPACED AT 2'-0" OC

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	4.0	1.25	2.00
C, D, E, G, H, I					
C TMVW+w	MT20	2.0	4.0		
F TTW+p	MT20	4.0	4.0	2.25	2.00
J TMVW+p	MT20	4.0	4.0	1.25	2.00
K BMV1+p	MT20	3.0	4.0		
L BMVW1-t	MT20	4.0	4.0		
M, N, O, P, Q					
M BMV1+w	MT20	2.0	4.0		
R BMVW1-t	MT20	4.0	4.0		
S BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	LC1 MAX (LBS)	UNBRACED LENGTH (LBS)
FR-TO				FR-TO			
S-B	-258 / 0	0.0	0.0 0.03 (1)	O-F	-124 / 0	0.09 (1)	
A-B	0 / 32	-84.3	-84.3 0.11 (1)	P-E	-189 / 0	0.08 (1)	
B-C	-50 / 0	-84.3	-84.3 0.11 (1)	Q-D	-172 / 0	0.04 (1)	
C-D	-19 / 0	-84.3	-84.3 0.04 (1)	R-C	-108 / 0	0.02 (1)	
D-E	-18 / 0	-84.3	-84.3 0.05 (1)	N-G	-191 / 0	0.08 (1)	
E-F	-25 / 0	-84.3	-84.3 0.05 (1)	M-H	-162 / 0	0.04 (1)	
F-G	-25 / 0	-84.3	-84.3 0.05 (1)	L-I	-162 / 0	0.03 (1)	
G-H	-17 / 0	-84.3	-84.3 0.05 (1)	B-R	0 / 28	0.01 (1)	
H-I	-24 / 0	-84.3	-84.3 0.04 (1)	L-J	0 / 32	0.01 (1)	
I-J	-21 / 0	-84.3	-84.3 0.04 (1)				
K-J	-84 / 0	0.0	0.0 0.01 (1)				
S-R	0 / 0	-28.0	-28.0 0.03 (2)				
R-Q	0 / 20	-28.0	-28.0 0.03 (2)				
O-P	0 / 16	-28.0	-28.0 0.02 (2)				
P-O	0 / 13	-28.0	-28.0 0.02 (2)				
O-N	0 / 13	-28.0	-28.0 0.02 (2)				
N-M	0 / 18	-28.0	-28.0 0.02 (2)				
M-L	0 / 20	-28.0	-28.0 0.02 (2)				
L-K	0 / 0	-28.0	-28.0 0.02 (2)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD	=	48.1	PSF	

SPACING = 24.0 IN. G/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11 (A-B:1), BC=0.03 (Q-R:2), WB=0.09 (F-O:1), SS=0.08 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	618	354	1887 822 2284 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (B) (INPUT = 0.90)
JSI METAL= 0.05 (G) (INPUT = 1.00)



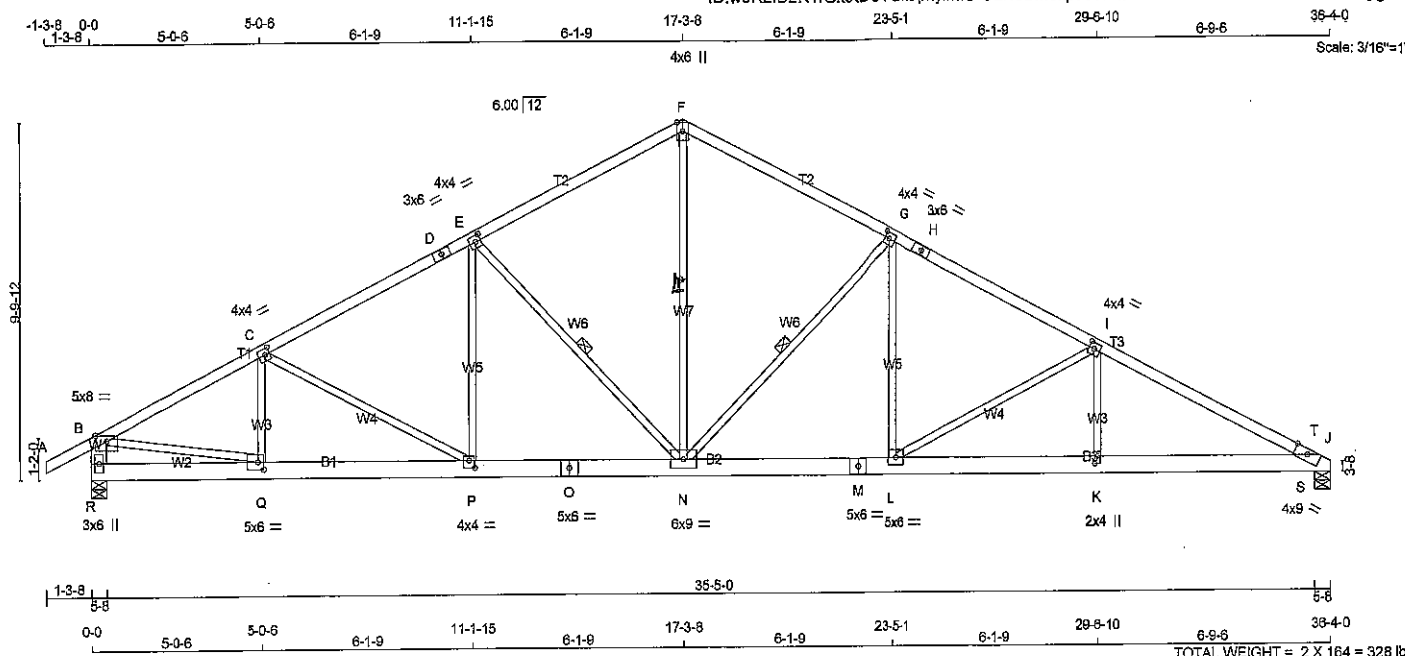
DWG NO. TAM 44089-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43854	DRWG NO.
285781	T16X	2	1	TRUSS DESC.		

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ID:w3K2IB2K4fGk9tDO7SkJphylfMC-ULN0BM5kpJe9PQ28HFnDsdzOYI23MNRBMFTNZDygrfR



TOTAL WEIGHT = 2 X 164 = 328 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - H	2x4	DRY	No.2	SPF	
H - J	2x4	DRY	No.2	SPF	
R - B	2x6	DRY	No.2	SPF	
R - O	2x6	DRY	No.2	SPF	
O - M	2x6	DRY	No.2	SPF	
M - J	2x6	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C, E, G, I						
C	TMVW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	6.0		
F	TTW-p	MT20	4.0	6.0	Edge	
H	TS-t	MT20	3.0	6.0		
J	TBM1-h	MT20	4.0	9.0	Edge	
K	BMV-w	MT20	2.0	4.0	2.50	1.00
L	BMVW-t	MT20	5.0	6.0		
M	BS-t	MT20	5.0	6.0		
N	BMVW-t	MT20	6.0	9.0		
O	BS-t	MT20	5.0	6.0		
P	BMVW-t	MT20	4.0	4.0	2.50	2.00
Q	BMVW-t	MT20	5.0	6.0	2.50	2.00
R	BMV1-p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
R	2154	0	2039	0
J	2039	0	2039	0

UNFACTORED REACTIONS

1ST CASE	MAX./MIN. COMPONENT REACTIONS	DEAD	SOIL
JT	COMBINED	SNOW	LIVE
R	1752	999 / 0	382 / 0
J	1674	930 / 0	382 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.12 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N, G-N. *FR-1*

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED UNBRACED LENGTH (FT)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED UNBRACED LENGTH (FT)
FR-TO					FR-TO			
A-B	0 / 26	-84.3	-84.3	0.11 (1)	Q-C	-283 / 73	0.08 (1)	10.00
B-C	-3052 / 0	-84.3	-84.3	0.40 (1)	C-P	-234 / 0	0.21 (1)	3.70
C-D	-2834 / 0	-84.3	-84.3	0.53 (1)	P-E	0 / 387	0.09 (2)	3.70
D-E	-2834 / 0	-84.3	-84.3	0.53 (1)	E-N	-818 / 0	0.43 (1)	3.70
E-F	-2230 / 0	-84.3	-84.3	0.48 (1)	N-F	0 / 1581	0.35 (1)	4.13
F-G	-2228 / 0	-84.3	-84.3	0.48 (1)	N-G	-1080 / 0	0.57 (1)	4.13
G-H	-3040 / 0	-84.3	-84.3	0.55 (1)	L-G	0 / 598	0.13 (2)	3.59
H-I	-3040 / 0	-84.3	-84.3	0.55 (1)	L-I	-933 / 0	0.85 (1)	3.59
I-T	-3848 / 0	-84.3	-84.3	0.65 (1)	K-I	0 / 408	0.08 (2)	3.12
T-J	-3927 / 0	-84.3	-84.3	0.24 (1)	B-Q	0 / 2780	0.63 (1)	3.41
R-B	-2079 / 0	0.0	0.0	0.13 (1)	S-T	-332 / 60	0.00 (1)	7.03
R-Q	0 / 0	-28.0	-28.0	0.08 (2)				10.00
Q-P	0 / 2747	-28.0	-28.0	0.40 (1)				10.00
P-O	0 / 2542	-28.0	-28.0	0.37 (1)				10.00
O-N	0 / 2542	-28.0	-28.0	0.37 (1)				10.00
N-M	0 / 2723	-28.0	-28.0	0.39 (1)				10.00
M-L	0 / 2723	-28.0	-28.0	0.39 (1)				10.00
L-K	0 / 3544	-28.0	-28.0	0.52 (1)				10.00
K-S	0 / 3544	-28.0	-28.0	0.82 (1)				10.00
S-J	0 / 3482	-28.0	-28.0	0.81 (1)				10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 25.6	PSF
	DL	= 3.0	PSF
BOT CH.	LL	= 10.5	PSF
	DL	= 7.0	PSF
TOTAL LOAD	= 46.1	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.21")
CALCULATED VERT. DEFL.(LL) = L/999 (0.21")
ALLOWABLE DEFL.(TL) = L/360 (1.21")
CALCULATED VERT. DEFL.(TL) = L/999 (0.35")

CSI: TC=0.65 (I-T:1), BC=0.62 (K-S:1), WB=0.85 (I-L:1), SSI=0.45 (J-S:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (Q) (INPUT = 0.90)
JSI METAL = 0.77 (J) (INPUT = 1.00)



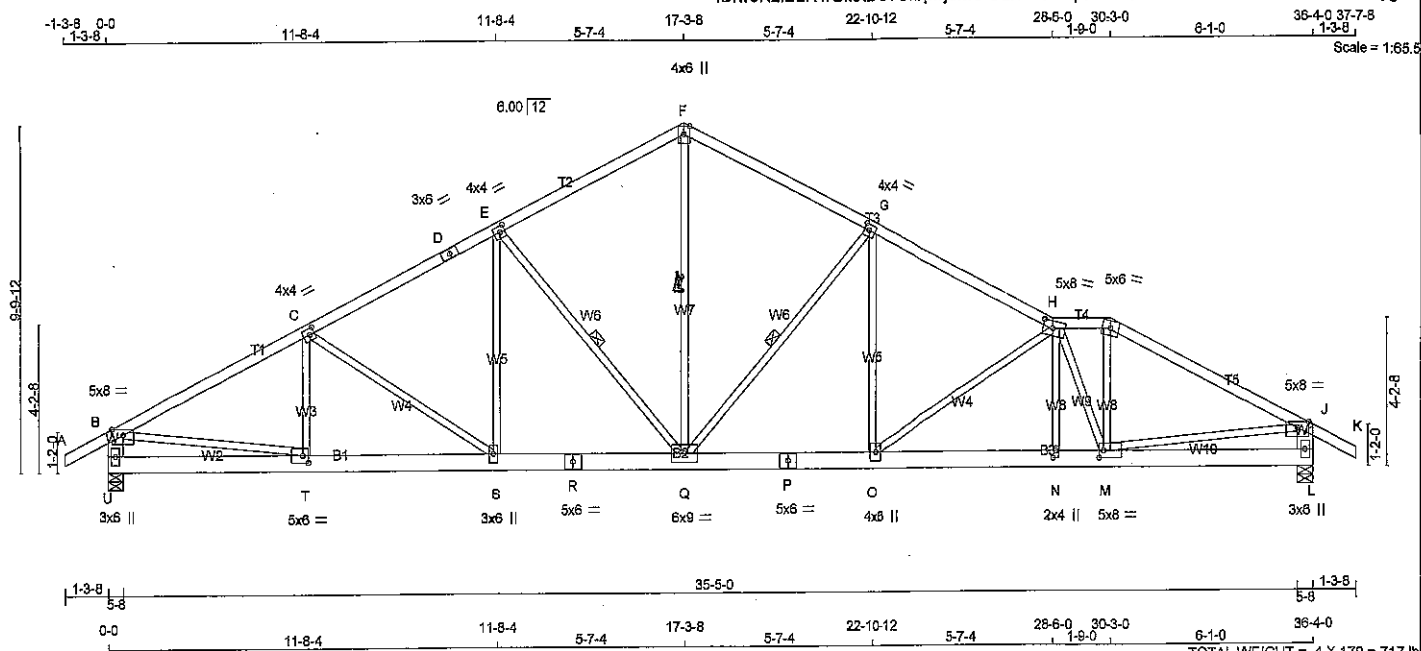
DRWG NO. TAM 4485977
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	49554	DRWG NO.
285781	T17S	4	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - H	2x4	DRY	No.2	SPF	
H - I	2x4	DRY	No.2	SPF	
I - K	2x4	DRY	No.2	SPF	
U - B	2x6	DRY	No.2	SPF	
L - J	2x6	DRY	No.2	SPF	
U - R	2x6	DRY	No.2	SPF	
R - P	2x6	DRY	No.2	SPF	
P - L	2x6	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TM/VW-p	MT20	5.0	8.0	Edge	
C, E, G					
C TM/VW-t	MT20	4.0	4.0	2.00	1.75
D TS-t	MT20	3.0	6.0		
F TTW-p	MT20	4.0	8.0	Edge	
H TTW/VW-m	MT20	5.0	8.0	2.50	3.50
I TTW-m	MT20	5.0	6.0		
J TM/VW-p	MT20	5.0	8.0	Edge	
L BMV1+p	MT20	3.0	6.0		
M BM/VW-t	MT20	5.0	8.0	2.50	1.50
N BM/VW-w	MT20	2.0	4.0	2.50	1.00
O BM/VW-t	MT20	4.0	6.0		
P BS-t	MT20	5.0	8.0		
Q BM/VW-t	MT20	6.0	9.0		
R BS-t	MT20	5.0	8.0		
S BM/VW-t	MT20	3.0	6.0		
T BM/VW-t	MT20	5.0	6.0	2.50	2.00
U BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	REGRD
U	2154	0	2154	0	0	5-8	5-8
L	2154	0	2154	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1752	999 / 0	382 / 0	0 / 0	0 / 0	371 / 0	0 / 0
L	1752	999 / 0	382 / 0	0 / 0	0 / 0	371 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.49 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-Q, G-Q. *PR*

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)
FR-TO					FR-TO			
A-B	0 / 26	-84.3	-84.3	0.11 (1)	10.00	T-C	-204 / 130	0.05 (1)
B-C	-3085 / 0	-84.3	-84.3	0.58 (1)	3.54	S-E	0 / 412	0.09 (2)
C-D	-2782 / 0	-84.3	-84.3	0.44 (1)	3.82	E-Q	-799 / 0	0.40 (1)
D-E	-2782 / 0	-84.3	-84.3	0.44 (1)	3.82	Q-F	0 / 1600	0.36 (1)
E-F	-2224 / 0	-84.3	-84.3	0.40 (1)	4.22	Q-G	-1073 / 0	0.54 (1)
F-G	-2226 / 0	-84.3	-84.3	0.42 (1)	4.17	O-G	0 / 600	0.14 (2)
G-H	-2952 / 0	-84.3	-84.3	0.49 (1)	3.64	Q-H	-719 / 0	0.62 (1)
H-I	-2761 / 0	-84.3	-84.3	0.11 (1)	4.11	N-H	0 / 82	0.02 (3)
I-J	-3040 / 0	-84.3	-84.3	0.60 (1)	3.49	H-M	-1297 / 0	0.36 (1)
J-K	0 / 28	-84.3	-84.3	0.11 (1)	10.00	M-I	0 / 1033	0.23 (1)
U-B	-2089 / 0	0.0	0.0	0.13 (1)	7.05	B-T	0 / 2801	0.63 (1)
L-J	-2070 / 0	0.0	0.0	0.13 (1)	7.05	M-J	0 / 2738	0.62 (1)
						C-S	-348 / 0	0.20 (1)
U-T	0 / 0	-28.0	-28.0	0.11 (2)	10.00			
T-S	0 / 2777	-28.0	-28.0	0.39 (1)	10.00			
S-R	0 / 2487	-28.0	-28.0	0.35 (1)	10.00			
R-Q	0 / 2487	-28.0	-28.0	0.35 (1)	10.00			
Q-P	0 / 2664	-28.0	-28.0	0.37 (1)	10.00			
P-O	0 / 2664	-28.0	-28.0	0.37 (1)	10.00			
O-N	0 / 3258	-28.0	-28.0	0.47 (1)	10.00			
N-M	0 / 3259	-28.0	-28.0	0.43 (1)	10.00			
M-L	0 / 0	-28.0	-28.0	0.12 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD	=	46.1	PSF	

SPACING = 24.0 IN./C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (1.21")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.20")
ALLOWABLE DEFL.(TL) = $L/360$ (1.21")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.33")

CSI: TC=0.80 (I-J), BC=0.47 (N-O), WB=0.83 (B-T), SS=0.21 (B-C-T)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PLI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 622 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (M) (INPUT = 0.90)
JSI METAL= 0.70 (H) (INPUT = 1.00)

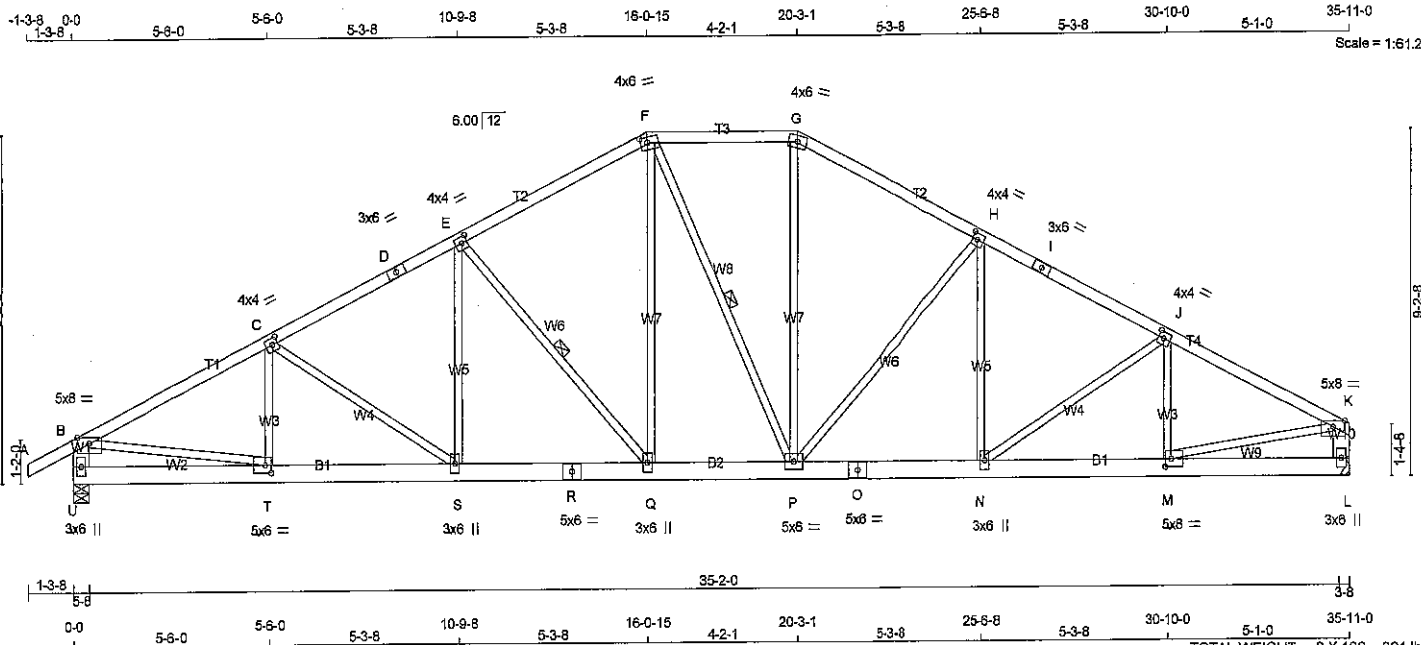


DWG NO. TAM 448697
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
285781	T18A	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:w3K2IB2K4fGk9ID07SkJphylfMC-yYxOOBMadm00adLryISPqWcG9VA5pvKbJCx5gygffQ



LUMBER			
N. L. G. A. RULES	SIZE	DRY	LUMBER
CHORDS			
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - K	2x4	DRY	No.2
U - B	2x6	DRY	No.2
L - K	2x6	DRY	No.2
U - R	2x6	DRY	No.2
R - O	2x6	DRY	No.2
O - L	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
DRY, SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-p	MT20	5.0	6.0	Edge
C, E, H, J					
C	TMVW-t	MT20	4.0	4.0	2.00 1.75
D	TS-t	MT20	3.0	6.0	
F	TTWW-m	MT20	4.0	6.0	1.75 2.25
G	TTW-m	MT20	4.0	6.0	
I	TS-t	MT20	3.0	6.0	
K	TMVW-p	MT20	5.0	6.0	Edge
L	BMV1-p	MT20	3.0	6.0	
M	BMVW-t	MT20	5.0	6.0	2.50 2.00
N, Q, S					
N	BMVW-t	MT20	3.0	6.0	
O	BS-t	MT20	5.0	6.0	
P	BMVW-t	MT20	5.0	6.0	
R	BS-t	MT20	5.0	6.0	
T	BMVW-t	MT20	5.0	6.0	2.50 2.00
U	BMV1-p	MT20	3.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
U	2130	0	0	5-8
L	2016	0	0	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
U	1733	988 / 0	377 / 0	0 / 0	367 / 0
L	1655	919 / 0	377 / 0	0 / 0	359 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.65 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-Q, F-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED CSI (LC)	MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED CSI (LC)
FR-TO		FROM TO		FR-TO		FROM TO	
A-B	0 / 26	-84.3 -84.3	0.11 (1)	10.00	T-C	-250 / 86	0.08 (1)
B-C	-3023 / 0	-84.3 -84.3	0.46 (1)	3.65	C-S	-252 / 0	0.18 (1)
C-D	-2807 / 0	-84.3 -84.3	0.38 (1)	3.86	S-E	0 / 377	0.08 (2)
D-E	-2807 / 0	-84.3 -84.3	0.38 (1)	3.86	E-Q	-738 / 0	0.33 (1)
E-F	-2284 / 0	-84.3 -84.3	0.38 (1)	4.22	Q-F	0 / 672	0.15 (1)
F-G	-2027 / 0	-84.3 -84.3	0.24 (1)	4.54	F-P	-13 / 0	0.01 (1)
G-H	-2279 / 0	-84.3 -84.3	0.38 (1)	4.22	P-G	0 / 658	0.15 (1)
H-I	-2740 / 0	-84.3 -84.3	0.38 (1)	3.90	P-H	-880 / 0	0.82 (1)
I-J	-2740 / 0	-84.3 -84.3	0.38 (1)	3.90	H-N	0 / 311	0.07 (2)
J-K	-2831 / 0	-84.3 -84.3	0.38 (1)	3.84	N-J	-113 / 0	0.08 (1)
U-B	-2049 / 0	0.0 0.0	0.13 (1)	7.07	M-J	-360 / 20	0.08 (1)
L-K	-1941 / 0	0.0 0.0	0.13 (1)	7.22	B-T	0 / 2247	0.82 (1)
U-T	0 / 0	-28.0 -28.0	0.10 (2)	10.00	M-K	0 / 269	0.58 (1)
T-S	0 / 2720	-28.0 -28.0	0.38 (1)	10.00			
S-R	0 / 2511	-28.0 -28.0	0.34 (1)	10.00			
R-Q	0 / 2511	-28.0 -28.0	0.34 (1)	10.00			
Q-P	0 / 2033	-28.0 -28.0	0.29 (1)	10.00			
P-O	0 / 2454	-28.0 -28.0	0.34 (1)	10.00			
O-N	0 / 2454	-28.0 -28.0	0.34 (1)	10.00			
N-M	0 / 2548	-28.0 -28.0	0.35 (1)	10.00			
M-L	0 / 0	-28.0 -28.0	0.09 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) =	L/360 (1.20")
CALCULATED VERT. DEFL.(LL) =	U/999 (0.16")
ALLOWABLE DEFL.(TL) =	L/360 (1.20")
CALCULATED VERT. DEFL.(TL) =	U/999 (0.26")

CSI: TC=0.46 (B-C:1), BC=0.38 (S-T:1), WB=0.92 (H-P:1), SSI=0.19 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2264 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (M) (INPUT = 0.90)
JSI METAL= 0.88 (T) (INPUT = 1.00)

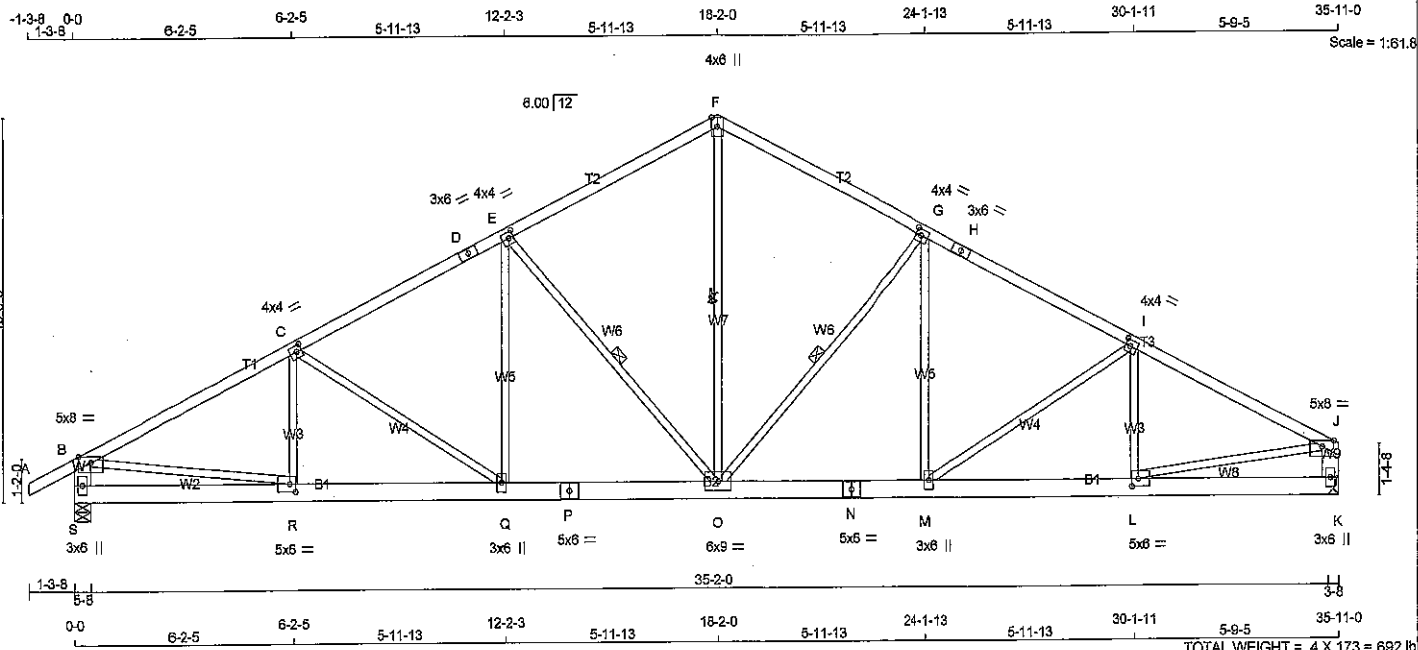


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
285781	T19A	4	1	TRUSS DESC.		

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ID:w3K2IB2K4fGk9tD07SkJphylfMC-yYxOOi6Madm00adLrylSPqWaf9U35uaKbJCx5gygfFq



TOTAL WEIGHT = 4 X 173 = 692 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - H	2x4	DRY	No.2	SPF	
H - J	2x4	DRY	No.2	SPF	
S - B	2x6	DRY	No.2	SPF	
K - J	2x6	DRY	No.2	SPF	
S - P	2x6	DRY	No.2	SPF	
P - N	2x6	DRY	No.2	SPF	
N - K	2x6	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table in inches)	JT TYPE	PLATES	W	LEN	Y	X
A - D	TMVW-p	MT20	5.0	8.0	Edge	
C, E, G, I						
D	TMVW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	6.0		
F	TTW+p	MT20	4.0	6.0	Edge	
H	TS-t	MT20	3.0	6.0		
J	TMVW-p	MT20	5.0	8.0	Edge	
K	BMV1+p	MT20	3.0	6.0		
L	BMVW-t	MT20	5.0	6.0	2.50	2.00
M	BMVW-t	MT20	3.0	6.0		
N	BS-t	MT20	5.0	6.0		
O	BMVW-t	MT20	6.0	9.0		
P	BS-t	MT20	5.0	6.0		
Q	BMVW-t	MT20	3.0	6.0		
R	BMVW-t	MT20	5.0	6.0	2.50	2.00
S	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
S	2130	0	2130	0
K	2016	0	2016	0

HANGER BY OTHERS
MIN. SEAT SIZE: 3-8

UNFACTORED REACTIONS

	1ST CASE	MAX/MIN	COMPONENT REACTIONS			
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
S	1733	988 / 0	377 / 0	0 / 0	0 / 0	367 / 0
K	1655	919 / 0	377 / 0	0 / 0	0 / 0	359 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.52 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-O, G-O. *f-o*

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

	CHORDS	WEBS
	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO	FROM TO	FR-TO
A-B	0 / 28	-84.3 -84.3 0.11 (1)
B-C	-3050 / 0	-84.3 -84.3 0.59 (1)
C-D	-2699 / 0	-84.3 -84.3 0.47 (1)
D-E	-2699 / 0	-84.3 -84.3 0.47 (1)
E-F	-2085 / 0	-84.3 -84.3 0.44 (1)
F-G	-2085 / 0	-84.3 -84.3 0.44 (1)
G-H	-2652 / 0	-84.3 -84.3 0.47 (1)
H-I	-2652 / 0	-84.3 -84.3 0.47 (1)
I-J	-2881 / 0	-84.3 -84.3 0.49 (1)
S-B	-2045 / 0	0.0 0.0 0.13 (1)
K-J	-1937 / 0	0.0 0.0 0.13 (1)
S-R	0 / 0	-28.0 -28.0 0.12 (2)
R-Q	0 / 2747	-28.0 -28.0 0.39 (1)
Q-P	0 / 2414	-28.0 -28.0 0.35 (1)
P-O	0 / 2414	-28.0 -28.0 0.35 (1)
O-N	0 / 2374	-28.0 -28.0 0.34 (1)
N-M	0 / 2374	-28.0 -28.0 0.34 (1)
M-L	0 / 2595	-28.0 -28.0 0.37 (1)
L-K	0 / 0	-28.0 -28.0 0.10 (2)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 3.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 46.1	PSF

SPACING = 24.0 IN G/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.17")
ALLOWABLE DEFL.(TL) = L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.27")

CSI: TC=0.59 (B-C:1), BC=0.39 (Q-R:1), WB=0.62 (B-R:1), SSI=0.22 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

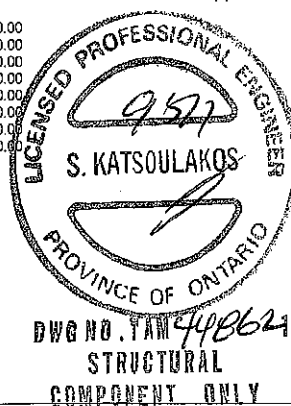
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 622 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

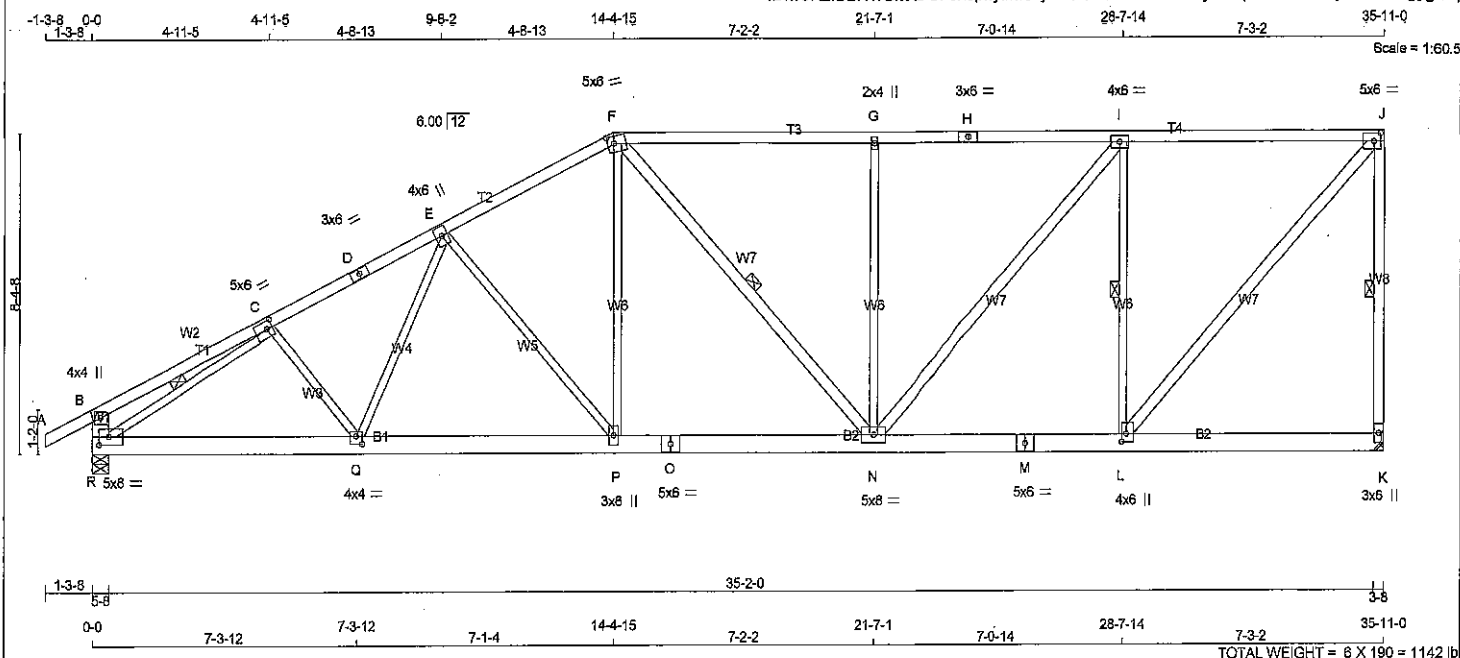
JSI GRIP= 0.89 (Q) (INPUT = 0.90)
JSI METAL= 0.69 (R) (INPUT = 1.00)



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43854	DRWG NO.
285781	T20A	6	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:w3K2IB2K4fGk9tD07SkJphylfMC-yYxOOi6Madm00adLrylSPqVWV39Uc5qvKbJcX5gygfFq



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - H	2x4	DRY	No.2	SPF	
H - J	2x4	DRY	No.2	SPF	
K - J	2x4	DRY	No.2	SPF	
R - B	2x6	DRY	No.2	SPF	
R - O	2x6	DRY	No.2	SPF	
O - M	2x6	DRY	No.2	SPF	
M - K	2x6	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	
L - J	2x4	DRY	No.2	SPF	
F - N	2x4	DRY	No.2	SPF	
N - I	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)							
JT	TYPE	PLATES	W	LEN	Y	X	
B	TMV+p	MT20	4.0	4.0			
C	TMVW-t	MT20	5.0	6.0	2.50	2.00	
D	TS-t	MT20	3.0	6.0			
E	TMVW-t	MT20	4.0	6.0			
F	TMVW-m	MT20	5.0	6.0	2.25	2.00	
G	TMVW-w	MT20	2.0	4.0			
H	TS-t	MT20	3.0	6.0			
I	TMVW-t	MT20	4.0	6.0			
J	TMVW-t	MT20	5.0	6.0	2.50	2.25	
K	BMV1+p	MT20	3.0	6.0			
L	BMVW-t	MT20	4.0	6.0	2.50	1.50	
M	BS-t	MT20	5.0	6.0			
N	BMVWW-t	MT20	5.0	6.0			
O	BS-t	MT20	5.0	6.0			
P	BMVW-t	MT20	3.0	6.0			
Q	BMVW-t	MT20	4.0	4.0	2.50	2.00	
R	BMVW-t	MT20	5.0	6.0	2.50	3.25	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX		
K	2016	0	2016	0	0				
R	2130	0	2130	0	0	5-8	5-8		

UNFACTORED REACTIONS		1ST LCASE		MAX./MIN. COMPONENT REACTIONS		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM. LIVE							
K	1655	919 / 0	377 / 0	0 / 0	0 / 0	0 / 0	359 / 0	0 / 0	359 / 0	0 / 0	0 / 0
R	1733	988 / 0	377 / 0	0 / 0	0 / 0	0 / 0	357 / 0	0 / 0	357 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.60 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF J-K, C-R, F-N, I-L

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MAX. FACTORED		FACTORED		MAX. FACTORED				
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX	MAX. UNBRAC	MEMB.	FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM	TO	LENGTH	FR-TO			
A-B	0 / 26	-84.3	-84.3	0.11 (1)	10.00	C-Q	-12 / 156	0.04 (3)
B-C	0 / 15	-84.3	-84.3	0.24 (1)	10.00	Q-E	0 / 311	0.07 (2)
C-D	-2959 / 0	-84.3	-84.3	0.44 (1)	3.70	E-P	-571 / 0	0.61 (1)
D-E	-2959 / 0	-84.3	-84.3	0.44 (1)	3.70	P-F	0 / 648	0.15 (1)
E-F	-2463 / 0	-84.3	-84.3	0.38 (1)	4.05	R-C	-3181 / 0	0.74 (1)
F-G	-2231 / 0	-84.3	-84.3	0.85 (1)	3.65	L-J	0 / 2278	0.37 (1)
G-H	-2231 / 0	-84.3	-84.3	0.88 (1)	3.60	F-N	-5 / 56	0.01 (1)
H-I	-2231 / 0	-84.3	-84.3	0.86 (1)	3.60	L-I	-1457 / 0	0.62 (1)
I-J	-1531 / 0	-84.3	-84.3	0.75 (1)	4.32	N-G	-653 / 0	0.88 (1)
K-J	-1933 / 0	0.0	0.0	0.59 (1)	4.84	N-I	0 / 1058	0.17 (1)
R-B	-284 / 0	0.0	0.0	0.02 (1)	7.81			
R-Q	0 / 2655	-28.0	-28.0	0.42 (1)	10.00			
Q-P	0 / 2559	-28.0	-28.0	0.38 (1)	10.00			
P-O	0 / 2194	-28.0	-28.0	0.34 (1)	10.00			
O-N	0 / 2194	-28.0	-28.0	0.34 (1)	10.00			
N-M	0 / 1531	-28.0	-28.0	0.30 (2)	10.00			
M-L	0 / 1531	-28.0	-28.0	0.30 (2)	10.00			
L-K	0 / 0	-28.0	-28.0	0.17 (3)	10.00			

957

LICENSED PROFESSIONAL ENGINEER

S. KATSOULAKOS

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.20")
CALCULATED VERT. DEFL. (LL) = L/999 (0.15")
ALLOWABLE DEFL. (TL) = L/360 (1.20")
CALCULATED VERT. DEFL. (TL) = L/998 (0.25")

CSI: TC=0.86 (G-I), BC=0.42 (Q-R:1), WB=0.86 (G-N:1), SS=0.28 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

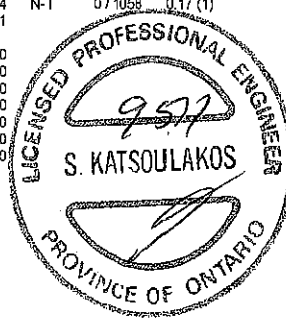
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PS) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 822 2284 1656

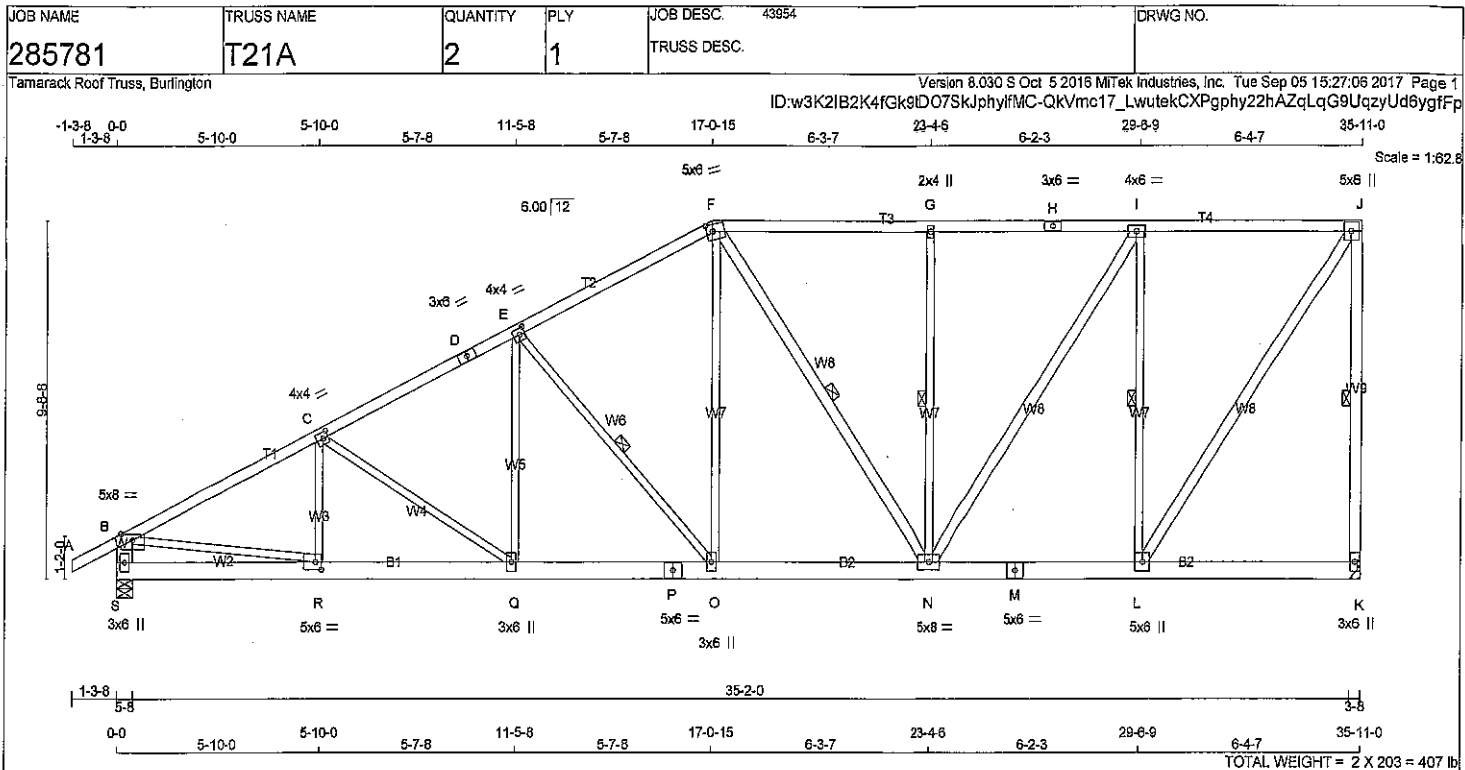
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (R) (INPUT = 0.90)
JSI METAL= 0.79 (C) (INPUT = 1.00)



DWG NO. TAM 4486317
STRUCTURAL
COMPONENT ONLY



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
D - F	2x4	DRY No.2	SPF
F - H	2x4	DRY No.2	SPF
H - J	2x4	DRY No.2	SPF
K - J	2x4	DRY No.2	SPF
S - B	2x6	DRY No.2	SPF
S - P	2x6	DRY No.2	SPF
P - M	2x6	DRY No.2	SPF
M - K	2x6	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
L - J 2x4 DRY No.2 SPF
F - N 2x4 DRY No.2 SPF
N - I 2x4 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.75
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMW-w	MT20	2.0	4.0		
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	4.0	6.0		
J	TMVW+hp	MT20	5.0	6.0		
K	BMV1+p	MT20	3.0	6.0		
L	BMWW+hp	MT20	5.0	6.0		
M	BS-t	MT20	5.0	6.0		
N	BMWW+hp	MT20	5.0	6.0		
O	BMWW+hp	MT20	3.0	6.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWW+hp	MT20	3.0	6.0		
R	BMWW-t	MT20	5.0	6.0	2.50	2.00
S	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	VERT	HORZ	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
K	2016	0	2016	0	0
S	2130	0	2130	0	0

MIN. SEAT SIZE: 3-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	1655	619 / 0	377 / 0	0 / 0	0 / 0	359 / 0	0 / 0
S	1733	988 / 0	377 / 0	0 / 0	0 / 0	367 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.50 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF J-K, E-Q, F-N, I-L, G-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAO CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-84.3 -84.3	0.11 (1)	10.00	R-C	-214 / 119	0.05 (1)
B-C	-3039 / 0	-84.3 -84.3	0.65 (1)	3.50	C-Q	-323 / 0	0.27 (1)
C-D	-2756 / 0	-84.3 -84.3	0.59 (1)	3.89	Q-E	0 / 404	0.09 (2)
D-E	-2756 / 0	-84.3 -84.3	0.59 (1)	3.69	E-O	-797 / 0	0.40 (1)
E-F	-2188 / 0	-84.3 -84.3	0.51 (1)	4.12	O-F	0 / 777	0.17 (1)
F-G	-1803 / 0	-84.3 -84.3	0.56 (1)	4.35	B-R	0 / 2759	0.62 (1)
G-H	-1803 / 0	-84.3 -84.3	0.57 (1)	4.32	L-J	0 / 2091	0.34 (1)
H-I	-1803 / 0	-84.3 -84.3	0.57 (1)	4.32	F-N	-257 / 0	0.17 (1)
I-J	-1179 / 0	-84.3 -84.3	0.52 (1)	5.16	L-I	-1525 / 0	0.62 (1)
K-J	-1943 / 0	0.0 0.0	0.84 (1)	4.83	N-G	-571 / 0	0.35 (1)
S-B	-2047 / 0	0.0 0.0	0.13 (1)	7.08	N-I	0 / 1131	0.18 (1)
S-R	0 / 0	-28.0 -28.0	0.11 (2)	10.00			
R-Q	0 / 2735	-28.0 -28.0	0.39 (1)	10.00			
Q-P	0 / 2465	-28.0 -28.0	0.34 (1)	10.00			
P-O	0 / 2465	-28.0 -28.0	0.34 (1)	10.00			
O-N	0 / 1947	-28.0 -28.0	0.28 (1)	10.00			
N-M	0 / 1179	-28.0 -28.0	0.23 (2)	10.00			
M-L	0 / 1179	-28.0 -28.0	0.23 (2)	10.00			
L-K	0 / 0	-28.0 -28.0	0.13 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL)= L/999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL)= L/999 (0.25")

CS1: TO=0.84 (J-K:1), BO=0.39 (Q-R:1), WB=0.92 (I-L:1), SS=0.25 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

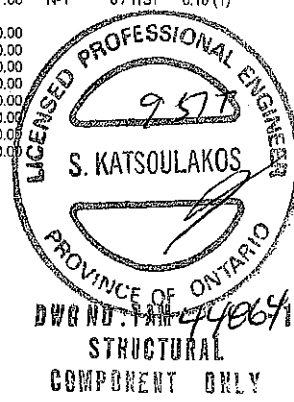
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2264 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (J) (INPUT = 0.90)
JSI METAL= 0.68 (R) (INPUT = 1.00)



Scale = 1:59.8

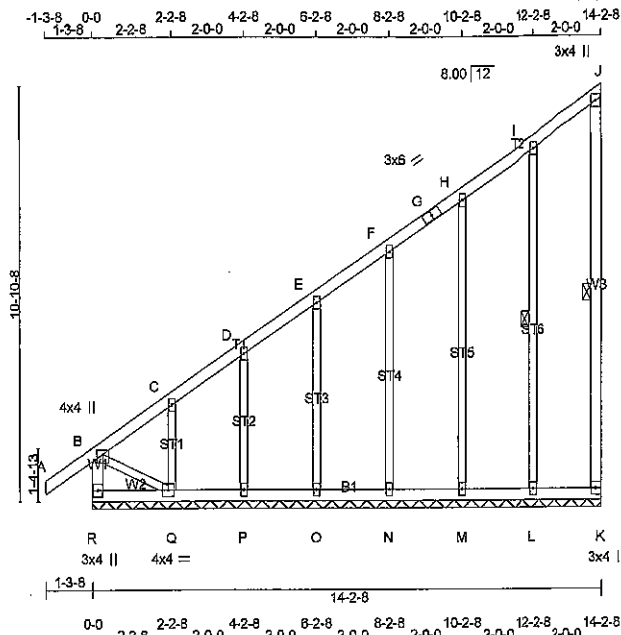


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
285781	G22	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1:60.6

TOTAL WEIGHT = 2 X 80 = 159 lb

LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER		
A - G	2x4	DRY	No.2	SPF	
G - J	2x4	DRY	No.2	SPF	
K - J	2x4	DRY	No.2	SPF	
R - B	2x4	DRY	No.2	SPF	
R - K	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
ALL GABLE WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	4.0	4.0	1.25 2.00
C, D, E, F, H, I					
C	TMW+w	MT20	2.0	4.0	
G	TS-t	MT20	3.0	5.0	
J	TMV+p	MT20	3.0	4.0	
K	BMV1+p	MT20	3.0	4.0	
L, M, N, O, P					
L	BMW1+w	MT20	2.0	4.0	
Q	BMWW1-t	MT20	4.0	4.0	
R	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF J-K, I-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0/32	-84.3 -84.3	0.11 (1)	10.00	L-I	-189 / 0	0.11 (1)
B-C	-11 / 0	-84.3 -84.3	0.05 (1)	6.25	M-H	-185 / 0	0.22 (1)
C-D	-17 / 0	-84.3 -84.3	0.05 (1)	6.25	N-F	-157 / 0	0.13 (1)
D-E	-9 / 0	-84.3 -84.3	0.04 (1)	10.00	O-E	-188 / 0	0.08 (1)
E-F	-7 / 0	-84.3 -84.3	0.04 (1)	10.00	P-D	-159 / 0	0.04 (1)
F-G	-4 / 0	-84.3 -84.3	0.04 (1)	10.00	Q-C	-194 / 0	0.03 (1)
G-H	-4 / 0	-84.3 -84.3	0.04 (1)	10.00	B-Q	0 / 20	0.00 (1)
H-I	0 / 0	-84.3 -84.3	0.04 (1)	10.00			
I-J	-9 / 0	-84.3 -84.3	0.04 (1)	10.00			
K-J	-72 / 0	0.0 0.0	0.04 (1)	6.25			
R-B	-206 / 0	0.0 0.0	0.02 (1)	7.81			
R-Q	0 / 0	-28.0 -28.0	0.03 (2)	10.00			
Q-P	0 / 11	-28.0 -28.0	0.03 (2)	10.00			
P-O	0 / 8	-28.0 -28.0	0.02 (2)	10.00			
O-N	0 / 5	-28.0 -28.0	0.02 (2)	10.00			
N-M	0 / 3	-28.0 -28.0	0.02 (2)	10.00			
M-L	0 / 2	-28.0 -28.0	0.03 (2)	10.00			
L-K	0 / 0	-28.0 -28.0	0.03 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11 (A-B:1), BC=0.03 (P-Q:2), WB=0.22 (H-M:1), SSI=0.08 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 622 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (B) (INPUT = 0.90)
JSI METAL= 0.05 (C) (INPUT = 1.00)



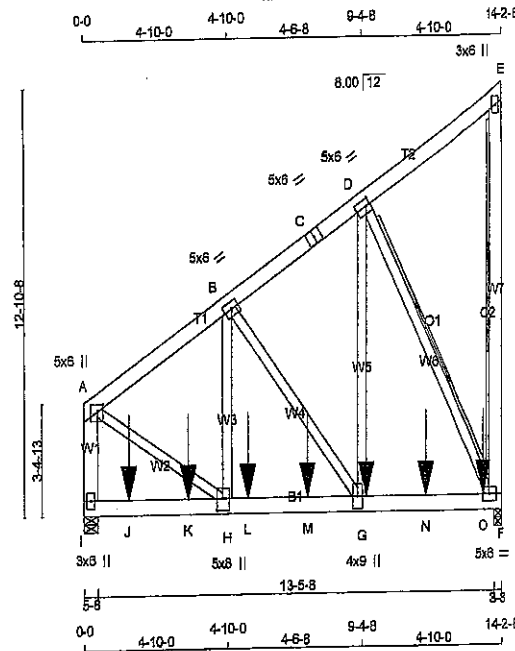
DWG NO. TAM 448017
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285298	T23	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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Scale = 1:70.1

TOTAL WEIGHT = 3 X 124 = 372 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF		
C - E	2x6	DRY	No.2	SPF		
E - F	2x6	DRY	No.2	SPF		
I - A	2x6	DRY	No.2	SPF		
I - F	2x6	DRY	2100F 1.8E	SPF		

ALL WEBS 2x4 DRY No.2 SPF EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C 2 12	TOP	
C - E 2 12	TOP	
E - F 2 12	SIDE(33.7)	
I - A 2 12	TOP	
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
I - F 2 7	SIDE(597.2)	
WEBS : (0.122"x3") SPIRAL NAILS		
2x4 1 6		

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	2.50	2.25
B	TMVW-l	MT20	5.0	6.0		
C	TS-l	MT20	5.0	6.0		
D	TMVW-l	MT20	5.0	6.0	2.50	1.50
E	TMV+p	MT20	3.0	6.0		
F	BMVW-l	MT20	5.0	6.0		
G	BMVW+l	MT20	4.0	9.0		
H	BMVW+l	MT20	5.0	6.0	4.25	2.50
I	BMV+l	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1987.9 lbs FACTORED DOWN AT 1-6-4, 1987.9 lbs FACTORED DOWN AT 3-6-4, 1987.9 lbs FACTORED DOWN AT 5-6-4, 1987.9 lbs FACTORED DOWN AT 7-6-4, 1987.9 lbs FACTORED DOWN AT 9-6-4, AND 1987.9 lbs FACTORED DOWN AT 11-6-4, AND 1992.3 lbs FACTORED DOWN AT 13-6-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	UP	IN-SX
F	8147	0	8147	0
I	7387	0	7387	0

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS
JT COMBINED	SNOW LIVE PERMLIVE WIND DEAD SOIL
F	6647 3759 / 0 1468 / 0 0 / 0 1420 / 0 0 / 0
I	6010 3400 / 0 1326 / 0 0 / 0 0 / 0 1283 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.89 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x6 DRY SPF No.2 T-BRACE AT E-F, D-F

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

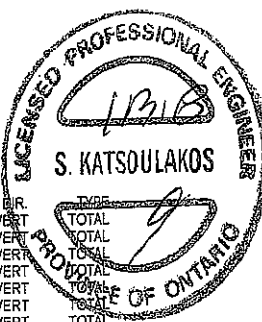
LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED MEMB. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRACED LENGTH	WEBS	MAX. FACTORED MEMB. FORCE (LBS)	MAX. UNBRACED LENGTH
FR-TO					FR-TO		
A-B	-5384 / 0	-84.3	-84.3 0.08 (1)	5.89	H-B	0 / 2809	0.14 (1)
B-C	-3573 / 0	-84.3	-84.3 0.08 (1)	6.25	B-G	-2453 / 0	0.54 (1)
C-D	-3573 / 0	-84.3	-84.3 0.08 (1)	6.25	G-D	0 / 1719	0.38 (1)
D-E	-25 / 0	-84.3	-84.3 0.05 (1)	6.25	D-F	-8417 / 0	0.82 (1)
E-F	-159 / 0	0.0	0.0 0.03 (1)	7.81	A-H	0 / 5200	0.28 (1)
F-A	-5858 / 0	0.0	0.0 0.21 (1)	7.20			
I-J	0 / 0	-28.0	-28.0 0.27 (1)	10.00			
J-K	0 / 0	-28.0	-28.0 0.27 (1)	10.00			
K-H	0 / 0	-28.0	-28.0 0.27 (1)	10.00			
H-L	0 / 4480	-28.0	-28.0 0.34 (1)	10.00			
L-M	0 / 4480	-28.0	-28.0 0.34 (1)	10.00			
M-G	0 / 4480	-28.0	-28.0 0.34 (1)	10.00			
G-N	0 / 2998	-28.0	-28.0 0.31 (1)	10.00			
N-O	0 / 2998	-28.0	-28.0 0.31 (1)	10.00			
O-F	0 / 2998	-28.0	-28.0 0.31 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.
G	9-6-4	-1988	-1988		BACK	VERT
J	1-6-4	-1988	-1988		BACK	VERT
K	3-6-4	-1988	-1988		BACK	VERT
L	5-6-4	-1988	-1988		BACK	VERT
M	7-6-4	-1988	-1988		BACK	VERT
N	11-6-4	-1988	-1988		BACK	VERT
O	13-6-4	-1992	-1992		BACK	VERT



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL			46.1	PSF

SPACING = 240 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.47")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.06")
ALLOWABLE DEFL.(TL) = $L/360$ (0.47")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.09")

CSI: TC=0.21/1.00 (A-I:1), BC=0.34/1.00 (G-H:1),
WB=0.82/1.00 (D-F:1), SSI=0.54/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

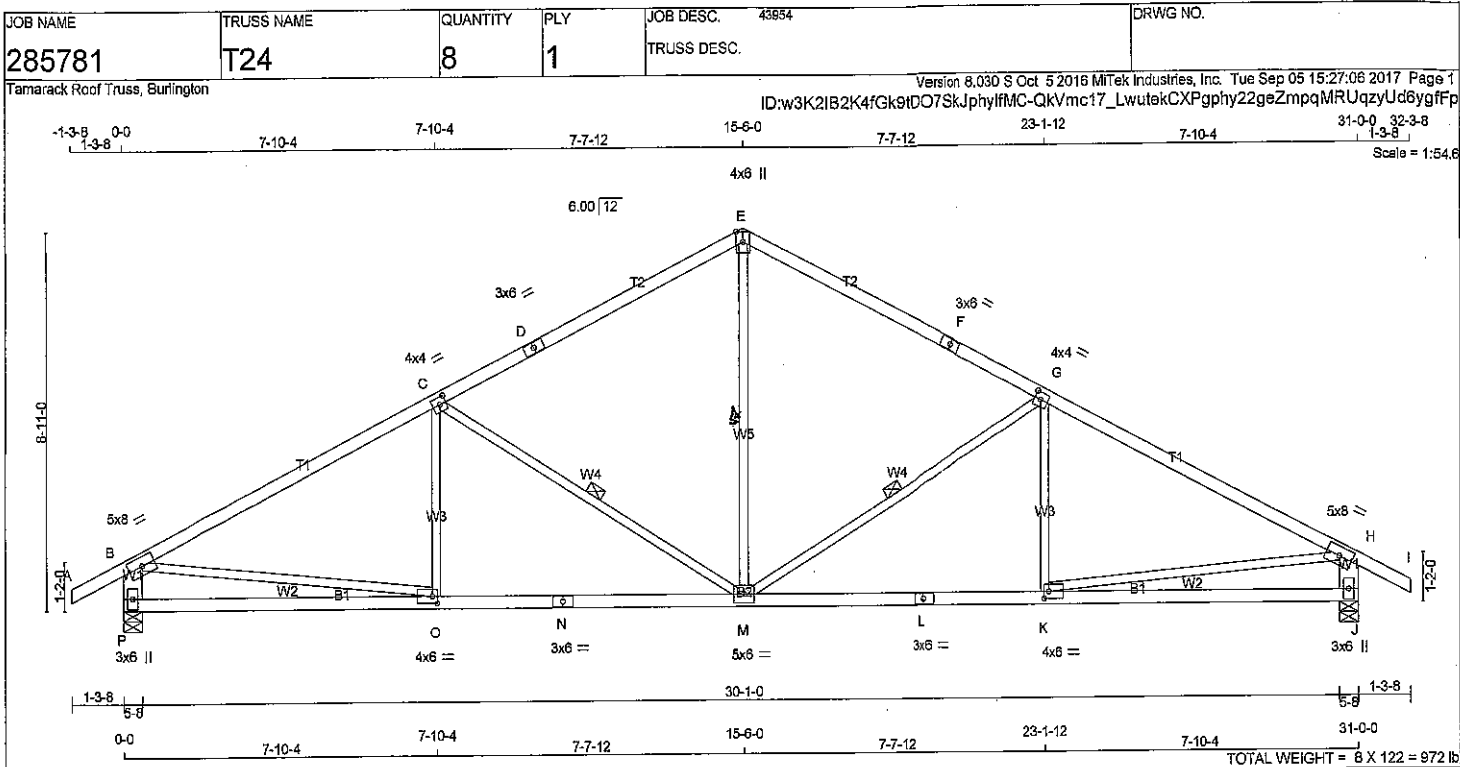
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1556

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (D) (INPUT = 0.90)
JSI METAL= 0.41 (G) (INPUT = 1.00)

DWG NO. TAM 3351-10
STRUCTURAL
COMPONENT ONLY



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
P - B	2x6	DRY	No.2
J - H	2x6	DRY	No.2
P - N	2x4	DRY	No.2
N - L	2x4	DRY	No.2
L - J	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0		
C	TMVW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	6.0		
E	TTW+p	MT20	4.0	6.0		Edge
F	TS-t	MT20	3.0	6.0		
G	TMVW-t	MT20	4.0	4.0	2.00	1.75
H	TMVW-t	MT20	5.0	8.0		
J	BMV1+p	MT20	3.0	6.0		
K	BMVW-t	MT20	4.0	6.0	2.00	1.50
L	BS-t	MT20	3.0	6.0		
M	BMVW-t	MT20	5.0	6.0		
N	BS-t	MT20	3.0	6.0		
O	BMVW-t	MT20	4.0	6.0	2.00	1.50
P	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
P	1854	0	1854	0	5-8	5-8
J	1854	0	1854	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
P	1506	863 / 0	326 / 0	0 / 0	0 / 0	316 / 0	0 / 0
J	1506	863 / 0	326 / 0	0 / 0	0 / 0	316 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.27 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-M, C-M. *E-M*

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS					
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)			
FR-TO		FROM	TO			FR-TO					
A-B	0 / 26	-84.3	-84.3	0.11 (1)	10.00	M-E	0 / 1098	0.25 (1)			
B-C	-2529 / 0	-84.3	-84.3	0.94 (1)	3.27	M-G	-840 / 0	0.48 (1)			
C-D	-1803 / 0	-84.3	-84.3	0.78 (1)	3.96	K-G	-9 / 307	0.07 (3)			
D-E	-1803 / 0	-84.3	-84.3	0.78 (1)	3.96	C-M	-840 / 0	0.48 (1)			
E-F	-1803 / 0	-84.3	-84.3	0.78 (1)	3.96	O-C	-9 / 307	0.07 (3)			
F-G	-1803 / 0	-84.3	-84.3	0.78 (1)	3.96	B-O	0 / 2306	0.52 (1)			
G-H	-2529 / 0	-84.3	-84.3	0.94 (1)	3.27	K-H	0 / 2306	0.52 (1)			
H-I	0 / 26	-84.3	-84.3	0.11 (1)	10.00						
P-B	-1764 / 0	0.0	0.0	0.11 (1)	7.48						
J-H	-1764 / 0	0.0	0.0	0.11 (1)	7.48						
P-O	0 / 0	-28.0	-28.0	0.42 (3)	10.00						
O-N	0 / 2293	-28.0	-28.0	0.68 (2)	10.00						
N-M	0 / 2293	-28.0	-28.0	0.86 (2)	10.00						
M-L	0 / 2293	-28.0	-28.0	0.68 (2)	10.00						
L-K	0 / 2293	-28.0	-28.0	0.68 (2)	10.00						
K-J	0 / 0	-28.0	-28.0	0.42 (3)	10.00						

A circular professional engineer seal. The outer ring contains the text "REGISTERED PROFESSIONAL ENGINEER" in a circular arrangement. In the center of the seal, the number "957" is handwritten in a large, bold, black ink.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/999 (0.18")
ALLOWABLE DEFL.(TL) = L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/999 (0.30")

CSI: TC=0.94 (G-H:1), BC=0.68 (K-M:2), WB=0.52 (H-K:1), SSI=0.29 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

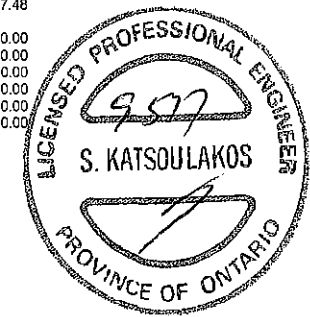
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 518 354 1667 822 2264 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (K) (INPUT = 0.90)
JSI METAL= 0.69 (L) (INPUT = 1.00)



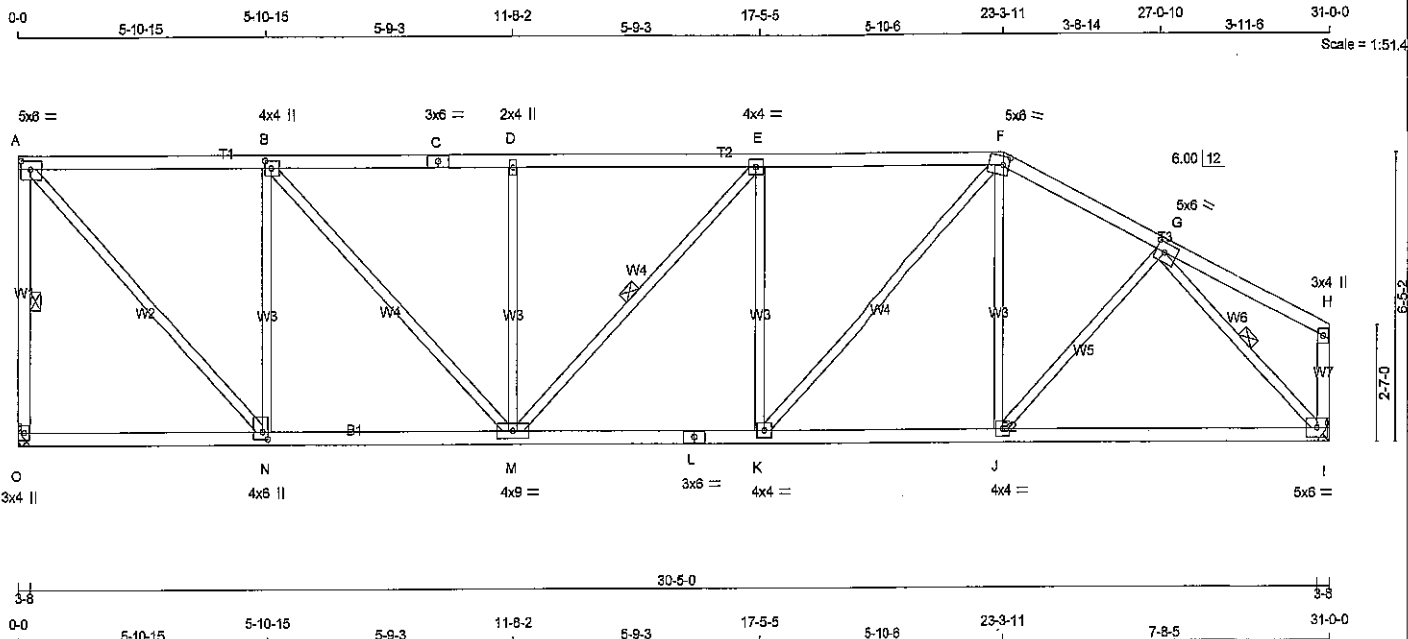
DWG NO. TAM 44667-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
285781	T26	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER				N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.	CHORDS	SIZE	LUMBER	DESCR.
O - A	2x4	DRY	No.2	SPF			
A - C	2x4	DRY	No.2	SPF			
C - F	2x4	DRY	No.2	SPF			
F - H	2x4	DRY	No.2	SPF			
I - H	2x4	DRY	No.2	SPF			
O - L	2x4	DRY	No.2	SPF			
L - I	2x4	DRY	No.2	SPF			
ALL WEBS	2x3	DRY	No.2	SPF			
EXCEPT							

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0	2.25	2.75
B	TMVW-t	MT20	4.0	4.0	2.00	1.75
C	TS-t	MT20	3.0	6.0		
D	TMVW-t	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	4.0		
F	TTWV-m	MT20	5.0	6.0	2.25	1.75
G	TMVW-t	MT20	5.0	6.0	2.50	2.50
H	TMV+p	MT20	3.0	4.0		
I	BMVW-t	MT20	5.0	6.0		
J	BMVW-t	MT20	4.0	4.0		
K	BMVW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMVW-t	MT20	4.0	9.0		
N	BMVW-t	MT20	4.0	6.0	2.00	1.50
O	BMV+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
O	1740	0	1740	0	0	0
I	1740	0	1740	0	0	0

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
O	1428	793 / 0	326 / 0	0 / 0	0 / 0	310 / 0	0 / 0	0 / 0
I	1429	793 / 0	326 / 0	0 / 0	0 / 0	310 / 0	0 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF A-O, G-I, E-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
O-A	-1673 / 0	0.0 0.0 0.32 (1)	5.13	J-F	-63 / 180	0.04 (1)	
A-B	-1421 / 0	-84.3 -84.3 0.49 (1)	4.86	J-G	0 / 398	0.09 (1)	
B-C	-2104 / 0	-84.3 -84.3 0.55 (1)	4.09	G-I	-2067 / 0	0.48 (1)	
C-D	-2104 / 0	-84.3 -84.3 0.55 (1)	4.09	N-B	-1290 / 0	0.66 (1)	
D-E	-2104 / 0	-84.3 -84.3 0.55 (1)	4.10	M-D	-447 / 0	0.30 (1)	
E-F	-2219 / 0	-84.3 -84.3 0.56 (1)	4.00	K-E	-434 / 2	0.29 (1)	
F-G	-1884 / 0	-84.3 -84.3 0.24 (1)	4.68	A-N	0 / 2048	0.46 (1)	
G-H	0 / 16	-84.3 -84.3 0.19 (1)	10.00	B-M	0 / 999	0.22 (1)	
H-I	-127 / 0	0.0 0.0 0.02 (1)	7.81	M-E	-168 / 0	0.08 (1)	
				K-F	0 / 797	0.18 (1)	
O-N	0 / 0	-28.0 -28.0 0.23 (3)	10.00				
N-M	0 / 1421	-28.0 -28.0 0.40 (2)	10.00				
M-L	0 / 2219	-28.0 -28.0 0.44 (1)	10.00				
L-K	0 / 2219	-28.0 -28.0 0.44 (1)	10.00				
K-J	0 / 1868	-28.0 -28.0 0.57 (2)	10.00				
J-I	0 / 1397	-28.0 -28.0 0.53 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6	PSF
DL	= 3.0	PSF
BOT CH. LL	= 10.5	PSF
DL	= 7.0	PSF
TOTAL LOAD	= 46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/999 (0.16")
ALLOWABLE DEFL.(TL) = L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/999 (0.27")

CSI: TC=0.56 (E-F:1), BC=0.57 (J-K:2), WB=0.86 (B-N:1), SSI=0.23 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

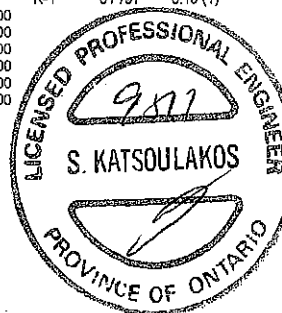
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1687
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (A) (INPUT = 0.90)
JSI METAL= 0.63 (L) (INPUT = 1.00)

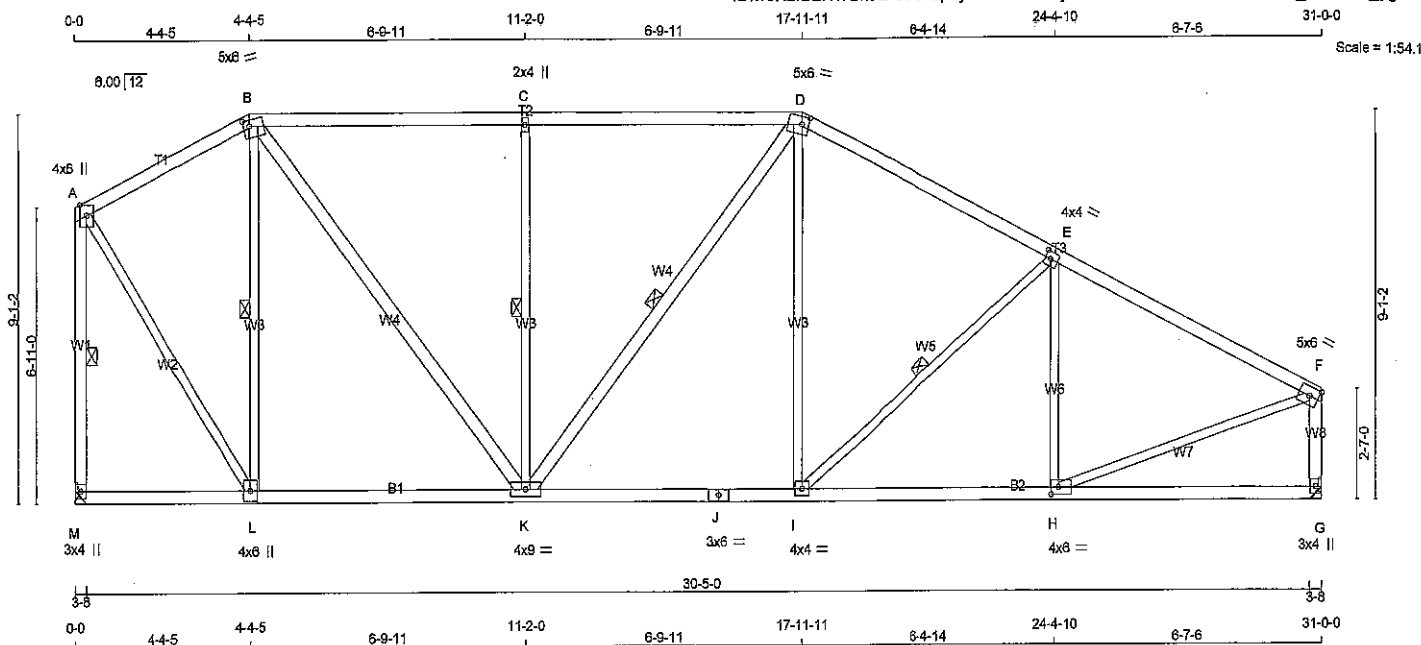


DRWG NO. TAM 44869.17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
285781	T28	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 151 = 302 lb
(M/F)

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
M - A	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
B - K	2x4	DRY	No.2	SPF
K - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
B	TTWW-m	MT20	5.0	6.0	1.75	1.75
C	TMVW+p	MT20	2.0	4.0		
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMVW-l	MT20	4.0	4.0	2.00	1.75
F	TMVW-l	MT20	5.0	6.0	Edge	
G	BMV1+p	MT20	3.0	4.0		
H	BMVW-l	MT20	4.0	6.0	2.00	2.25
I	BMVW-l	MT20	4.0	4.0		
J	BS-l	MT20	3.0	6.0		
K	BMVWV-l	MT20	4.0	9.0		
L	BMVWV-l	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
M	1740	0	1740	0	0			HANGER BY OTHERS	
G	1740	0	1740	0	0			MIN. SEAT SIZE: 3-8	
								HANGER BY OTHERS	
								MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1429	793 / 0	326 / 0	0 / 0	0 / 0	310 / 0	0 / 0
G	1429	793 / 0	326 / 0	0 / 0	0 / 0	310 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.28 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF B-L, C-K, D-K, E-L, A-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)	MAX. UNBRACED LENGTH (FT)
FR-TO				FR-TO			
A-B	-854 / 0	-84.3	-84.3 0.21 (1)	L-B	-957 / 0	0.52 (1)	6.25
B-C	-1478 / 0	-84.3	-84.3 0.55 (1)	B-K	0 / 1188	0.19 (1)	4.69
C-D	-1478 / 0	-84.3	-84.3 0.55 (1)	K-C	-710 / 0	0.38 (1)	4.69
D-E	-1710 / 0	-84.3	-84.3 0.52 (1)	K-D	-82 / 0	0.04 (2)	4.49
E-F	-1894 / 0	-84.3	-84.3 0.56 (1)	I-D	0 / 437	0.10 (2)	4.28
M-A	-1697 / 0	0.0	0.0 0.38 (1)	I-E	-288 / 0	0.14 (1)	5.10
G-F	-1684 / 0	0.0	0.0 0.22 (1)	H-E	-387 / 87	0.21 (1)	8.43
				A-L	0 / 1354	0.31 (1)	
				H-F	0 / 1819	0.41 (1)	
M-L	0 / 0	-28.0	-28.0 0.21 (2)				10.00
L-K	0 / 752	-28.0	-28.0 0.35 (2)				10.00
K-J	0 / 1506	-28.0	-28.0 0.44 (2)				10.00
J-I	0 / 1508	-28.0	-28.0 0.44 (2)				10.00
I-H	0 / 1720	-28.0	-28.0 0.49 (2)				10.00
H-G	0 / 0	-28.0	-28.0 0.29 (3)				10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	25.6	PSF
DL	=	3.0	PSF
BOT CH. LL	=	10.5	PSF
DL	=	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL)= L/999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL)= L/999 (0.17")

CSI: TC=0.56 (E-F:1), BC=0.49 (H-I:2), WB=0.52 (B-L:1), SS=0.28 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

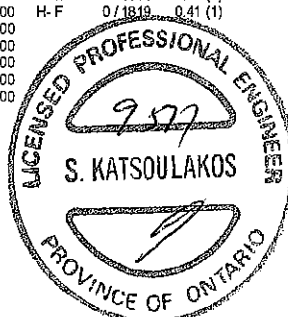
NAIL VALUES

PLATE GRIP(OR)Y	SHEAR (PSI)	SECTION (PL)
MT20	618	354 1687 822 2284 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (B) (INPUT = 0.90)
JSI METAL= 0.49 (F) (INPUT = 1.00)

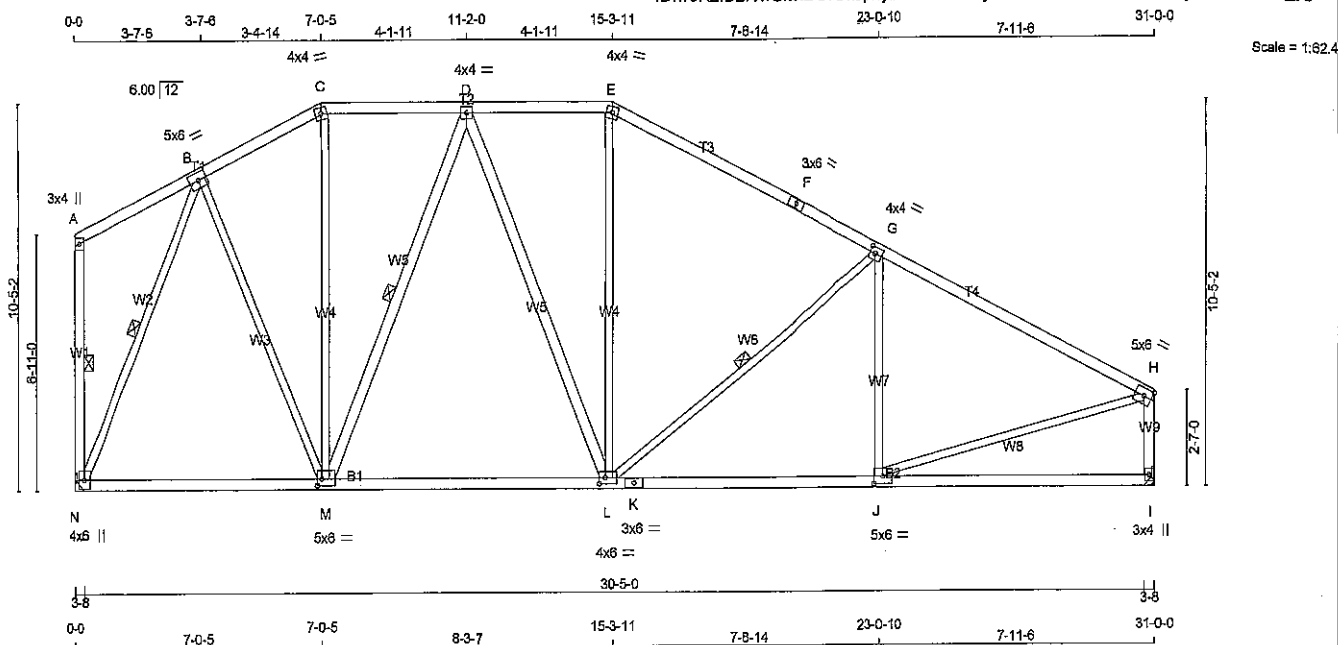


DWG NO. TAM 44871-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43854	DRWG NO.
285781	T29	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MITek Industries, Inc. Tue Sep 05 15:27:08 2017 Page 1
ID:w3K2IB2K4fGk3fDO7SkJphylfMC-M7cW1J8E1Y8at1LwW5r91T81tNSylAGmHHRbl_ygfFn



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
N - A	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF

ALL WEBS EXCEPT	SIZE	LUMBER	DESCR.
M - D	2x4	DRY	No.2
D - L	2x4	DRY	No.2

DRY, SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	5.0	6.0		
C	TTW-m	MT20	4.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TTW-m	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
H	TMWW-t	MT20	5.0	6.0		Edge
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	5.0	6.0	2.50	2.75
K	BS-t	MT20	3.0	6.0		
L	BMWW-t	MT20	4.0	6.0	2.00	2.00
M	BMWW-t	MT20	5.0	6.0	2.25	1.50
N	BMVW1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
N	1740	0	1740	0	0			HANGER BY OTHERS	
I	1740	0	1740	0	0			MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1429	793 / 0	326 / 0	0 / 0	0 / 0	310 / 0	0 / 0
I	1429	793 / 0	326 / 0	0 / 0	0 / 0	310 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.69 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-M, G-L, A-N, B-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	MAX. FACTORED FORCE (LBS)
FR-TO				FR-TO			
A-B	0 / 15	-84.3	-84.3 0.16 (1)	10.00	B-M	0 / 785	0.17 (1)
B-C	-1084 / 0	-84.3	-84.3 0.13 (1)	5.93	M-C	0 / 259	0.06 (2)
C-D	-958 / 0	-84.3	-84.3 0.19 (1)	6.13	M-D	-750 / 0	0.49 (1)
D-E	-1343 / 0	-84.3	-84.3 0.20 (1)	5.39	D-L	0 / 291	0.05 (1)
E-F	-1535 / 0	-84.3	-84.3 0.78 (1)	4.17	L-E	0 / 345	0.08 (2)
F-G	-1535 / 0	-84.3	-84.3 0.78 (1)	4.17	L-G	-557 / 0	0.41 (1)
G-H	-1949 / 0	-84.3	-84.3 0.86 (1)	3.69	J-G	-260 / 183	0.18 (1)
N-A	-117 / 0	0.0	0.0 0.03 (1)	6.25	N-B	-1679 / 0	0.95 (1)
I-H	-1650 / 0	0.0	0.0 0.22 (1)	6.45	J-H	0 / 1846	0.42 (1)
N-M	0 / 856	-28.0	-28.0 0.47 (2)	10.00			
M-L	0 / 1235	-28.0	-28.0 0.54 (2)	10.00			
L-K	0 / 1775	-28.0	-28.0 0.83 (2)	10.00			
K-J	0 / 1775	-28.0	-28.0 0.83 (2)	10.00			
J-I	0 / 0	-28.0	-28.0 0.43 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (1.03")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.18")
ALLOWABLE DEFL.(TL) = $L/360$ (1.03")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.27")

CSI: TC=0.89 (G-H:1), BC=0.83 (J-L:2), WB=0.95 (B-N:1), SS=0.29 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

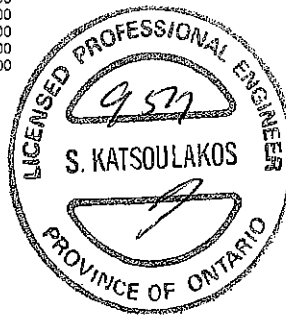
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1967
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (J) (INPUT = 0.90)
JSI METAL= 0.59 (K) (INPUT = 1.00)



DWG NO. T29 4487217
STRUCTURAL
COMPONENT ONLY



2

P	13-6-12	-10	-101	—	BACK	VERT	TOTAL	DWG NO. TAM
Q	1-6-12	-40	-70	—	BACK	VERT	TOTAL	
					BACK	VERT	TOTAL	

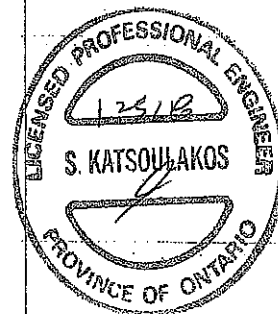
5900.13 1012

VERT	TOTAL
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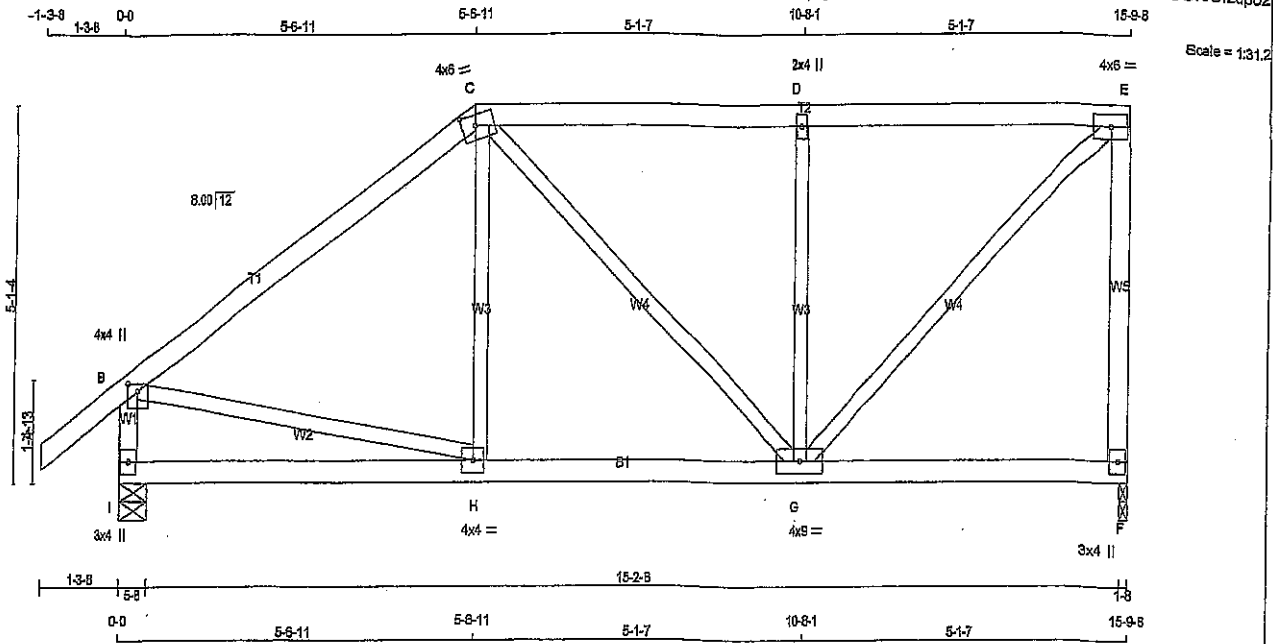
DWG NO. TAM /
STRUCT

JOB NAME 285864	TRUSS NAME T30	QUANTITY 1	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington Version 8.200 S Dec 12 2017 Mitak Industries, Inc. Thu Jan 25 08:11:38 2018 Page 2					
ID:w3K2IB2K4fGk9tD07SkJchylfMG-n9pxGxbL1J JDsRadQxQMImIGBPDh90gRFmLROzru7p					

FACTORED CONCENTRATED LOADS (LBS)						
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.
S	5-6-12	-40	-70	--	BACK	VERT
T	7-8-12	-40	-70	--	BACK	VERT
U	8-8-12	-40	-70	--	BACK	VERT
V	13-6-12	-40	-70	--	BACK	VERT
						TOTAL



DWG NO. TAM **5980**
 STRUCTURAL
 COMPONENT ONLY



TOTAL WEIGHT = 65 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTWW-m	MT20	4.0	6.0	1.75	2.50
D	TMVW-w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMVWW-t	MT20	4.0	9.0		
H	BMVW-t	MT20	4.0	4.0		
I	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ UPLIFT		
F	886	0	886	0	1-8	1-8
I	1002	0	1002	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
F	728	404/0	165/0	0/0	0/0	158/0	0/0
I	805	474/0	166/0	0/0	0/0	166/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, I

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.95 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)
FR-TO		FROM	TO	UNBRACED LENGTH	FR-TO		
A-B	0/32	-84.3	-84.3	0.11 (1)	10.00	H-C	0/222
B-C	-821/0	-84.3	-84.3	0.49 (1)	5.85	C-G	-23/33
C-D	-707/0	-84.3	-84.3	0.38 (1)	8.25	G-D	-532/0
D-E	-707/0	-84.3	-84.3	0.38 (1)	6.25	G-E	0/967
E-F	-826/0	0.0	0.0	0.26 (1)	7.81	B-H	0/695
F-E	-937/0	0.0	0.0	0.10 (1)	7.81		
I-H	0/0	-28.0	-28.0	0.19 (3)	10.00		
H-G	0/983	-28.0	-28.0	0.28 (2)	10.00		
G-F	0/0	-28.0	-28.0	0.16 (3)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 25.6 PSF
 DL = 3.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TFC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = 1/360 (0.53")
 CALCULATED VERT. DEFL.(LL) = 1/999 (0.05")
 ALLOWABLE DEFL.(TL) = 1/300 (0.53")
 CALCULATED VERT. DEFL.(TL) = 1/999 (0.07")

CSI: TC=0.49/1.00 (B-C:1), SC=0.28/1.00 (G-H:2),
 WS=0.22/1.00 (E-G:1), SS=0.21/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PL)
MT20	618	354	1657

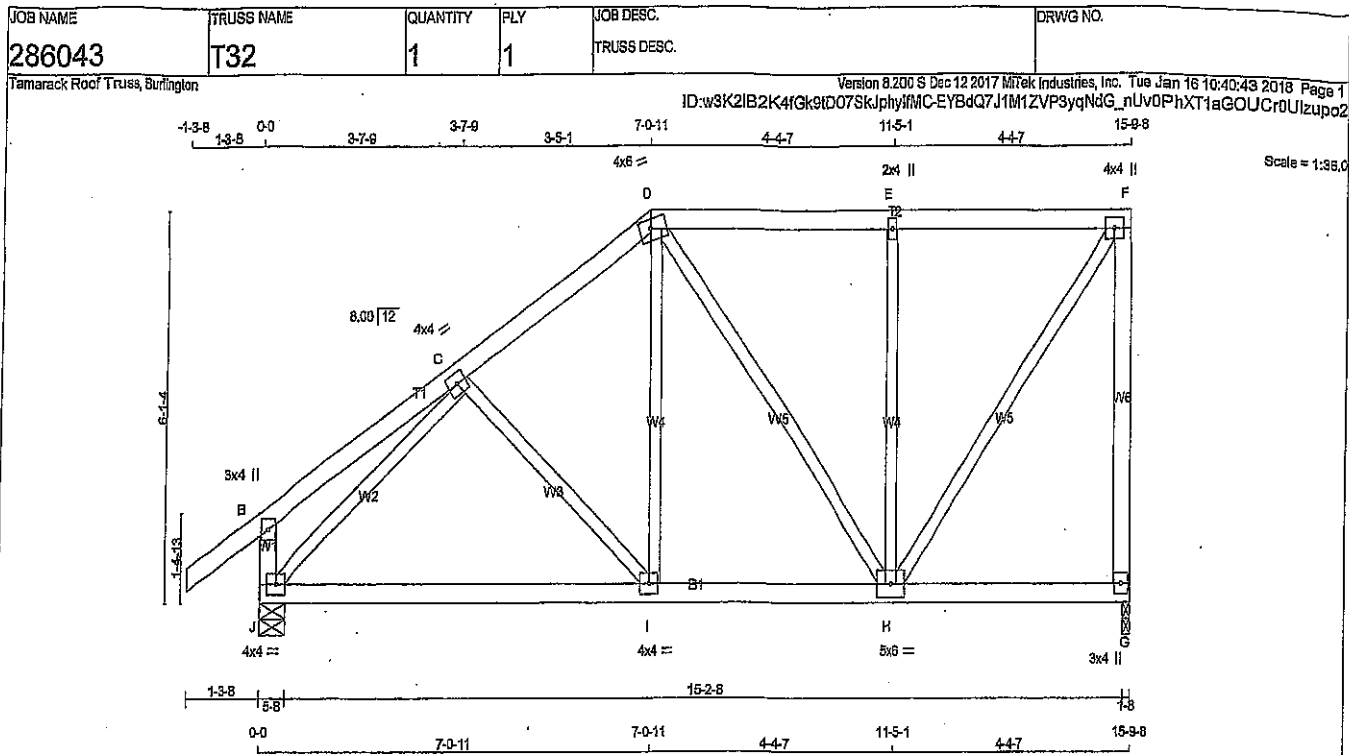
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (B) (INPUT = 0.60)
 JSI METAL= 0.28 (H) (INPUT = 1.00)



DWG NO. TAM 3996-18
 STRUCTURAL
 COMPONENT ONLY



LUMBER
N. L. G. A. RULES
CHORDS SIZE LUMBER DESCR.
A - D 2x4 DRY No.2 SPF
D - F 2x4 DRY No.2 SPF
G - F 2x4 DRY No.2 SPF
J - B 2x4 DRY No.2 SPF
J - G 2x4 DRY No.2 SPF
ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	7MMW+1	MT20	4.0	4.0		
D	TTVW+m	MT20	4.0	8.0	1.75	2.50
E	TMVW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	4.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMVWV+1	MT20	5.0	8.0		
I	BMVWV+1	MT20	4.0	4.0		
J	BMVWV+1	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	RECORD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
G	885	0	888	0	1-8	1-8
J	1002	0	1002	0	5-8	5-8

UNFACTORED REACTIONS

JT	COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
G	728	404 / 0	165 / 0	0 / 0	0 / 0	158 / 0	0 / 0
J	806	474 / 0	165 / 0	0 / 0	0 / 0	166 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LENGTH (LC)	MEMB.
FR-TO		FROM TO		FR-TO			
A-B	0 / 32	-84.3 -84.3	0.11 (1)	10.00	C-I	-127 / 62	0.05 (1)
B-C	0 / 18	-84.3 -84.3	0.16 (1)	10.00	I-D	0 / 248	0.08 (2)
C-D	-758 / 0	-84.3 -84.3	0.17 (1)	6.25	D-H	-188 / 0	0.17 (2)
D-E	-522 / 0	-84.3 -84.3	0.27 (1)	6.25	H-E	-453 / 0	0.27 (1)
E-F	-522 / 0	-84.3 -84.3	0.27 (1)	6.25	H-F	0 / 684	0.19 (1)
G-F	-631 / 0	0.0 0.0	0.60 (1)	7.81	J-C	-987 / 0	0.38 (1)
J-B	-231 / 0	0.0 0.0	0.02 (1)	7.81			
J-I	0 / 707	-28.0 -28.0	0.36 (2)	10.00			
I-H	0 / 619	-28.0 -28.0	0.36 (2)	10.00			
H-G	0 / 0	-28.0 -28.0	0.13 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(65 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.53")
CALCULATED VERT. DEFL.(LL) = L/689 (0.11")
ALLOWABLE DEFL.(TL) = L/360 (0.53")
CALCULATED VERT. DEFL.(TL) = L/989 (0.18")

CSI: TC=0.80/1.00 (F-G:1), BC=0.38/1.00 (H-I:2), WB=0.39/1.00 (C-J:1), SSI=0.18/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

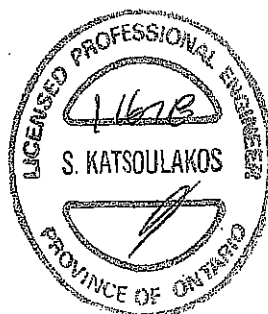
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1856

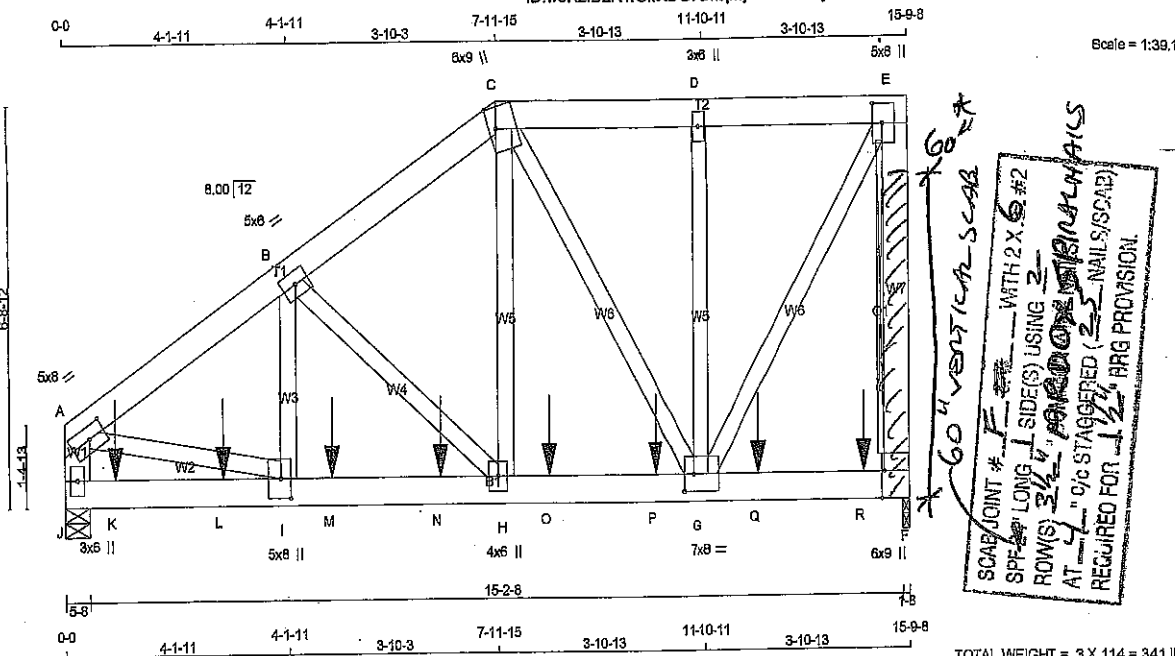
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.88 (F) (INPUT = 0.90)
JSI METAL= 0.95 (C) (INPUT = 1.00)



DWG NO. TAM 3997-18
STRUCTURAL
COMPONENT ONLY



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x8	DRY No.2	SPF
C - E	2x8	DRY No.2	SPF
E - F	2x8	DRY No.2	SPF
F - A	2x8	DRY No.2	SPF
J - F	2x8	DRY 2100F 1.8E	SPF
ALL WEBS	2x4	DRY No.2	SPF
EXCEPT			

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122'X3') SPIRAL NAILS			
A - C	2	12	TOP
C - E	2	12	TOP
E - F	2	12	TOP
J - A	2	12	TOP
BOTTOM CHORDS : (0.122'X3') SPIRAL NAILS			
J - F	2	8	SIDE (478.6)
WEBS : (0.122'X3') SPIRAL NAILS			
2x4	1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	X
A	TMVW-t	MT20	5.0	8.0	2.50 3.75
B	TMVW-t	MT20	5.0	6.0	
C	TTWW-t	MT20	6.0	9.0	4.25 1.50
D	TMVW-t	MT20	3.0	6.0	
E	TMVW-t	MT20	5.0	8.0	4.00 2.25
F	BMV1-t	MT20	8.0	9.0	Edge 0.50
G	BMVWVW-t	MT20	7.0	8.0	3.50 2.00
H	BMVWVW-t	MT20	4.0	6.0	
I	BMVWVW-t	MT20	5.0	8.0	4.00 2.25
J	BMV1-t	MT20	3.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS					
JT	VERT	HORIZ	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
F	7812	0	7812	0	1-8
J	7977	0	7977	0	5-8

UNFACTORED REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND
F	6361	3617 / 0	1392 / 0	0 / 0	0 / 0
J	6463	3729 / 0	1378 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.96 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x8 DRY SPF No.2 T-BRACE AT E-F

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 8" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. (CSI (LC))	MEMB.	FORCE (LBS)	MAX. FACTORED (CSI (LC))	
FR-TO		FROM	TO	FR-TO			
A-B	-8363 / 0	-84.3	-84.3 0.09 (1)	4.96	I-B	0 / 2609	0.15 (1)
B-C	-5978 / 0	-84.3	-84.3 0.07 (1)	5.68	B-H	-2674 / 0	0.28 (1)
C-D	-3902 / 0	-84.3	-84.3 0.04 (1)	6.25	H-C	0 / 4908	0.26 (1)
D-E	-3902 / 0	-84.3	-84.3 0.04 (1)	6.25	C-G	-2275 / 0	0.51 (1)
E-F	-6299 / 0	0.0	0.0 0.25 (1)	7.81	G-D	-283 / 16	0.04 (1)
J-A	-8335 / 0	0.0	0.0 0.14 (1)	6.99	G-E	0 / 7116	0.38 (1)
					A-I	0 / 7107	0.38 (1)
J-K	0 / 0	-28.0	-28.0 0.19 (1)	10.00			
K-L	0 / 0	-28.0	-28.0 0.19 (1)	10.00			
L-I	0 / 0	-28.0	-28.0 0.19 (1)	10.00			
I-M	0 / 6946	-28.0	-28.0 0.29 (1)	10.00			
M-N	0 / 6946	-28.0	-28.0 0.29 (1)	10.00			
N-H	0 / 6946	-28.0	-28.0 0.29 (1)	10.00			
H-O	0 / 5068	-28.0	-28.0 0.28 (1)	10.00			
O-P	0 / 5068	-28.0	-28.0 0.26 (1)	10.00			
P-G	-0.15088	-28.0	-28.0 0.26 (1)	10.00			
G-Q	0 / 293	-28.0	-28.0 0.18 (1)	10.00			
Q-R	0 / 293	-28.0	-28.0 0.18 (1)	10.00			
R-F	0 / 293	-28.0	-28.0 0.18 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)				FACE DIR			
JT	LOC.	LC1	MAX-	MAX+	FRONT	VERT	TOTAL
K	11-4	-1827	-1827	---	FRONT	VERT	TOTAL
L	2-11-4	-1828	-1828	---	FRONT	VERT	TOTAL
M	4-11-4	-1826	-1826	---	FRONT	VERT	TOTAL
N	6-11-4	-1712	-1712	---	FRONT	VERT	TOTAL
O	8-11-4	-1712	-1712	---	FRONT	VERT	TOTAL
P	10-11-4	-1712	-1712	---	FRONT	VERT	TOTAL
Q	12-11-4	-1712	-1712	---	FRONT	VERT	TOTAL
R	14-11-4	-1714	-1714	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.8 PSF

DL = 3.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(65% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $\frac{1}{380}$ (0.53")

CALCULATED VERT. DEFL.(LL) = $\frac{1}{999}$ (0.05")

ALLOWABLE DEFL.(TL) = $\frac{1}{380}$ (0.53")

CALCULATED VERT. DEFL.(TL) = $\frac{1}{999}$ (0.08")

CSI: TC=0.25/1.00 (E-F:1), BC=0.28/1.00 (H-I:1), WB=0.51/1.00 (C-G:1), SS=0.44/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

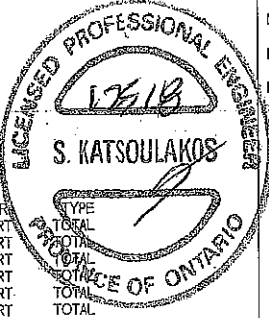
MT20 618 354 1657 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

ISI GRIP= 0.88 (G) (INPUT = 0.90)

ISI METAL= 0.80 (I) (INPUT = 1.00)



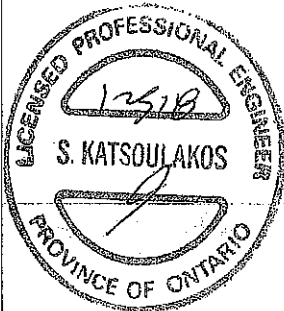
DWG NO. TAM 5981-18

STRUCTURAL

COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285748	T33	1	3	TRUSS DESC.	

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1827.1 lbs FACTORED DOWN AT 11-4, 1826.2 lbs FACTORED DOWN AT 2-11-4, 1826.2 lbs FACTORED DOWN AT 4-11-4, 1711.9 lbs FACTORED DOWN AT 6-11-4, 1711.9 lbs FACTORED DOWN AT 8-11-4, 1711.9 lbs FACTORED DOWN AT 10-11-4, AND 1711.9 lbs FACTORED DOWN AT 12-11-4, AND 1714.0 lbs FACTORED DOWN AT 14-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.



DWG NO. TAM 5981-18

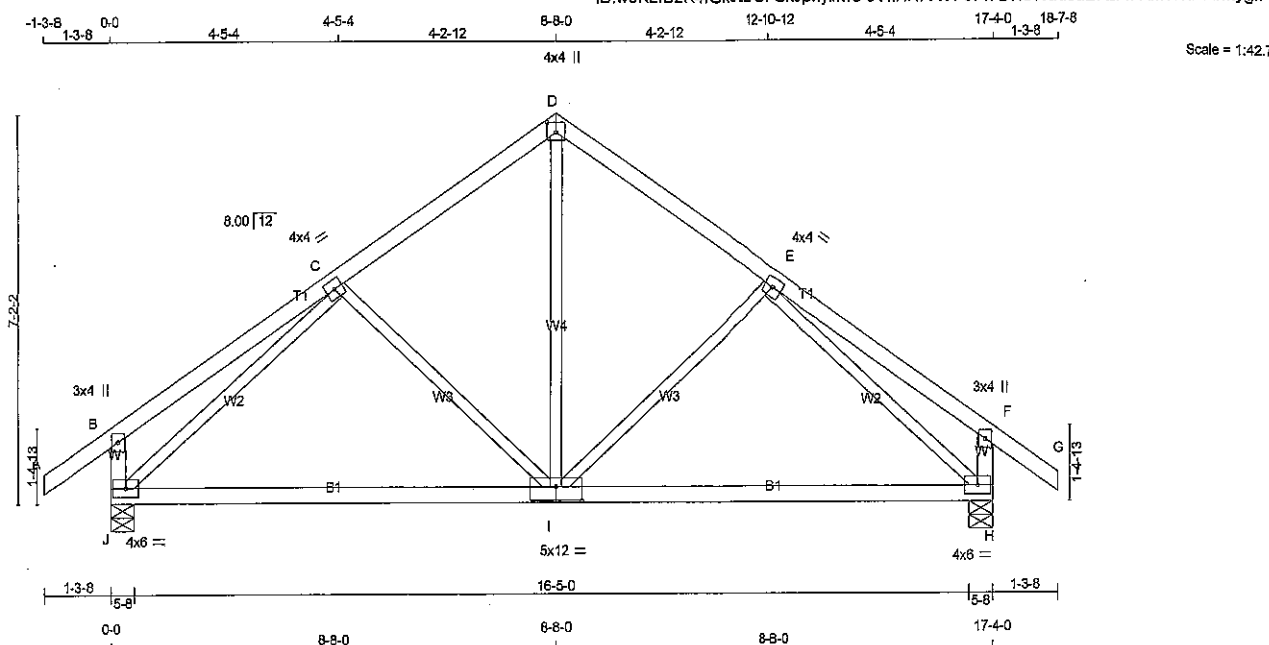
STRUCTURAL
COMPONENT ONLY

JOB NAME 285781	TRUSS NAME T34	QUANTITY 1	PLY 1	JOB DESC. 43954 TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:w3K2IB2K4fGk9td07SkJphylfMC-JVkhRPAPV9PI7LVleWud6uDXJA7Km9f3lwimtygff



TOTAL WEIGHT = 73 lb

LUMBER	CHORDS	SIZE	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF	
D - G	2x4	DRY	No.2	SPF	
J - B	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
J - I	2x4	DRY	No.2	SPF	
I - H	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW+p	MT20	4.0	4.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BSWWW-t	MT20	5.0	12.0	3.00	6.00
J	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

BEARINGS							
	FACTORED		MAXIMUM FACTORED			INPUT	REQRD
	GROSS REACTION		GROSS REACTION			BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	1089	0	1089	0	0	5-8	5-8
H	1089	0	1089	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	877	514 / 0	182 / 0	0 / 0	0 / 0	182 / 0	0 / 0
H	877	514 / 0	182 / 0	0 / 0	0 / 0	182 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	MAX. FACTORED TORSION (LBS-FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	MAX. FACTORED TORSION (LBS-FT)
FR-TO					FR-TO			
A-B	0 / 32	-84.3	-84.3	0.11 (1)	I-D	0 / 587	0.13 (1)	
B-C	0 / 23	-84.3	-84.3	0.25 (1)	I-E	-212 / 42	0.13 (1)	
C-D	-807 / 0	-84.3	-84.3	0.20 (1)	C-I	-212 / 42	0.13 (1)	
D-E	-807 / 0	-84.3	-84.3	0.20 (1)	J-C	-1097 / 0	0.63 (1)	
E-F	0 / 23	-84.3	-84.3	0.25 (1)	E-H	-1097 / 0	0.63 (1)	
F-G	0 / 32	-84.3	-84.3	0.11 (1)				
J-B	-255 / 0	0.0	0.0	0.03 (1)				
H-F	-255 / 0	0.0	0.0	0.03 (1)				
J-I	0 / 809	-28.0	-28.0	0.70 (2)				
I-H	0 / 809	-28.0	-28.0	0.70 (2)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN./G

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/380 (0.58")
CALCULATED VERT. DEFL.(LL) = L/999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.58")
CALCULATED VERT. DEFL.(TL) = L/766 (0.27")

CSI: TC=0.25 (E-F:1), BC=0.70 (H-I:2), WB=0.63 (E-H:1), SSI=0.21 (H:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 622 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (E) (INPUT = 0.90)
JSI METAL= 0.89 (I) (INPUT = 1.00)



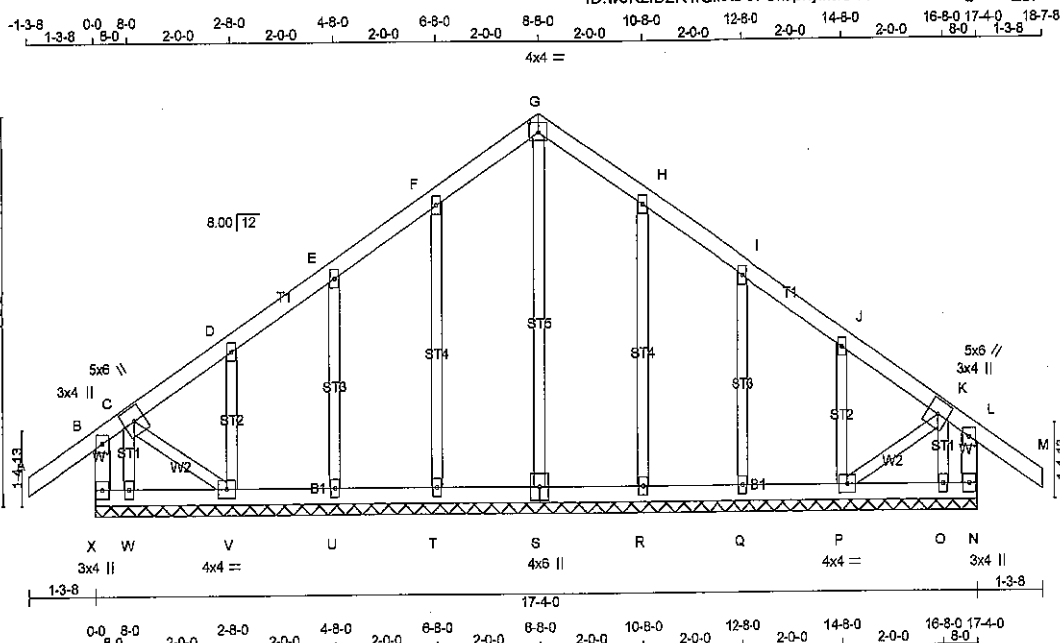
DWG NO. TAM 4487-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
285781	G34	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1:42.7

TOTAL WEIGHT = 80 lb

LUMBER	CHORDS	SIZE	DRY	LUMBER	DESCR.
N - L	G - A	2x4	DRY	No.2	SPF
A - G	M	2x4	DRY	No.2	SPF
G - M	B	2x4	DRY	No.2	SPF
X - B	L	2x4	DRY	No.2	SPF
N - L	S	2x4	DRY	No.2	SPF
X - S	N	2x4	DRY	No.2	SPF

ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW+t	MT20	5.0	6.0		
D, E, F, H, I, J						
D	TMW+w	MT20	2.0	4.0	2.25	2.00
G	TTW-p	MT20	4.0	4.0		
K	TMWW+t	MT20	5.0	6.0		
L	TMV+p	MT20	3.0	4.0		
N	BMV1+p	MT20	3.0	4.0		
O, Q, R, T, U, W						
O	BMW1+w	MT20	2.0	4.0		
P	BMWW1+t	MT20	4.0	4.0		
S	BSW1+t	MT20	4.0	6.0		
V	BMWW1+t	MT20	4.0	4.0		
X	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX. (LC)	MAX. UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. (LC)
FR-TO						FR-TO			
A-B	0/32	-84.3	-84.3	0.11 (1)	10.00	S-G	-117/0	0.10 (1)	
B-C	-59/0	-84.3	-84.3	0.11 (1)	6.25	T-F	-182/0	0.10 (1)	
C-D	-31/0	-84.3	-84.3	0.04 (1)	6.25	U-E	-160/0	0.05 (1)	
D-E	-30/0	-84.3	-84.3	0.04 (1)	6.25	V-D	-174/0	0.03 (1)	
E-F	-23/0	-84.3	-84.3	0.05 (1)	6.25	W-C	-37/0	0.01 (1)	
F-G	-31/0	-84.3	-84.3	0.05 (1)	6.25	R-H	-182/0	0.10 (1)	
G-H	-31/0	-84.3	-84.3	0.05 (1)	6.25	Q-I	-160/0	0.05 (1)	
H-I	-23/0	-84.3	-84.3	0.05 (1)	6.25	P-J	-174/0	0.03 (1)	
I-J	-30/0	-84.3	-84.3	0.04 (1)	6.25	O-K	-37/0	0.01 (1)	
J-K	-31/0	-84.3	-84.3	0.04 (1)	6.25	C-V	0/38	0.01 (1)	
K-L	-59/0	-84.3	-84.3	0.11 (1)	6.25	P-K	0/38	0.01 (1)	
L-M	0/32	-84.3	-84.3	0.11 (1)	10.00				
X-B	-247/0	0.0	0.0	0.03 (1)	7.81				
N-L	-247/0	0.0	0.0	0.03 (1)	7.81				
X-W	0/0	-28.0	-28.0	0.02 (2)	10.00				
W-V	-2/0	-28.0	-28.0	0.02 (2)	10.00				
V-U	0/24	-28.0	-28.0	0.03 (2)	10.00				
U-T	0/21	-28.0	-28.0	0.02 (2)	10.00				
T-S	0/18	-28.0	-28.0	0.02 (2)	10.00				
S-R	0/18	-28.0	-28.0	0.02 (2)	10.00				
R-Q	0/21	-28.0	-28.0	0.02 (2)	10.00				
Q-P	0/24	-28.0	-28.0	0.03 (2)	10.00				
P-O	-2/0	-28.0	-28.0	0.02 (2)	10.00				
O-N	0/0	-28.0	-28.0	0.02 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 085-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11 (A-B:1), BC=0.03 (P-Q:2), WB=0.10 (G-S:1), SSI=0.09 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

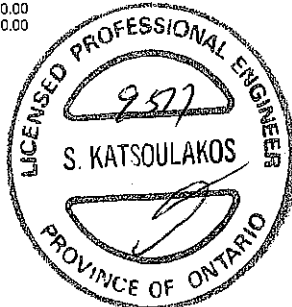
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.49 (G) (INPUT = 0.90)
JSI METAL= 0.05 (S) (INPUT = 1.00)



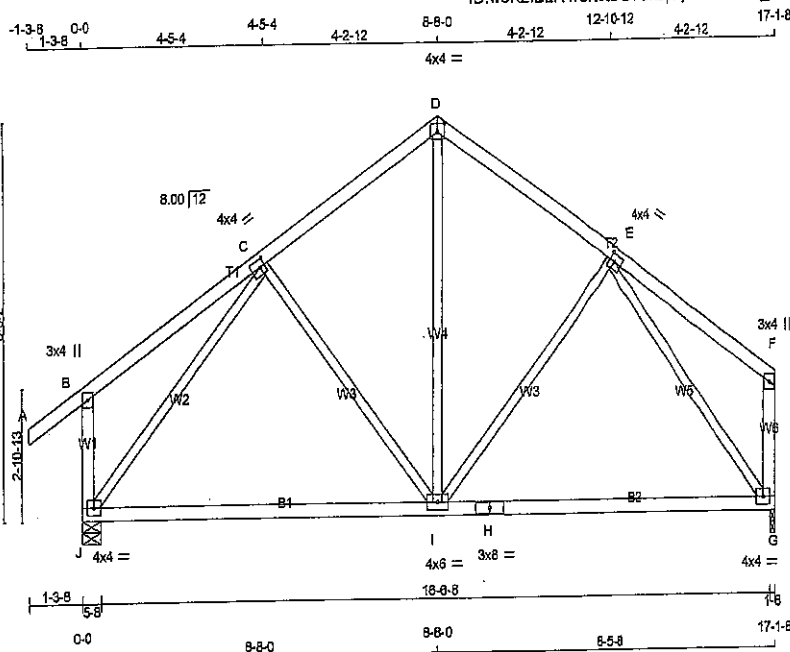
DWG NO. TAN 4489-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285748	T35A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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Scale = 1:50.8

TOTAL WEIGHT = 79 lb

(M/F)

LUMBER	DESOR.
N. L. G. A. RULES	
CHORDS	
A - D 2x4 DRY No.2	SPF
D - F 2x4 DRY No.2	SPF
J - B 2x4 DRY No.2	SPF
G - F 2x4 DRY No.2	SPF
J - H 2x4 DRY No.2	SPF
H - G 2x4 DRY No.2	SPF
ALL WEBS 2x3 DRY No.2	SPF
EXCEPT	
DRY: SEASONED LUMBER.	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	SRG
	VERT	HORZ	DOWN	HORZ
JT	1077	0	1077	0
G	991	0	991	0

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN.	COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	869	508 / 0	180 / 0	0 / 0	0 / 0	179 / 0	0 / 0
G	789	438 / 0	180 / 0	0 / 0	0 / 0	171 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED	FORCE	VERT. LOAD	LC1	MAX.	MAX.	MEMB.	MAX. FACTORED	FORCE	MAX.
			(LBS)			(PLF)	CSI (LC)			(LBS)	CSI (LC)
FR-TO				FROM	TO			FR-TO			
A-B	0 / 32	-84.3	-84.3	0.11	(1)	10.00		C-I	-89 / 90	0.08	(1)
B-C	0 / 24	-84.3	-84.3	0.26	(1)	10.00		I-D	0 / 438	0.10	(2)
C-D	-851 / 0	-84.3	-84.3	0.20	(1)	6.25		I-E	-59 / 101	0.05	(1)
D-E	-850 / 0	-84.3	-84.3	0.19	(1)	5.25		J-C	-931 / 0	0.83	(1)
E-F	0 / 24	-84.3	-84.3	0.25	(1)	10.00		E-G	-929 / 0	0.79	(1)
J-B	-254 / 0	0.0	0.0	0.04	(1)	7.81					
G-F	-130 / 0	0.0	0.0	0.02	(1)	7.81					
J-I	0 / 578	-28.0	-28.0	0.67	(2)	10.00					
I-H	0 / 559	-28.0	-28.0	0.65	(2)	10.00					
H-G	0 / 559	-28.0	-28.0	0.65	(2)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	25.6	PSF
DL	=	3.0	PSF
BOT CH. LL	=	10.5	PSF
DL	=	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.57")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.16")
ALLOWABLE DEFL.(TL) = $L/360$ (0.57")
CALCULATED VERT. DEFL.(TL) = $L/751$ (0.27")

CSI: TC=0.26/1.00 (B-C:1), BC=0.67/1.00 (I-J:2), WB=0.83/1.00 (C-J:1), SS=0.21/1.00 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1887
	822	2284	1556

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (E) (INPUT = 0.90)
JSI METAL= 0.45 (H) (INPUT = 1.00)



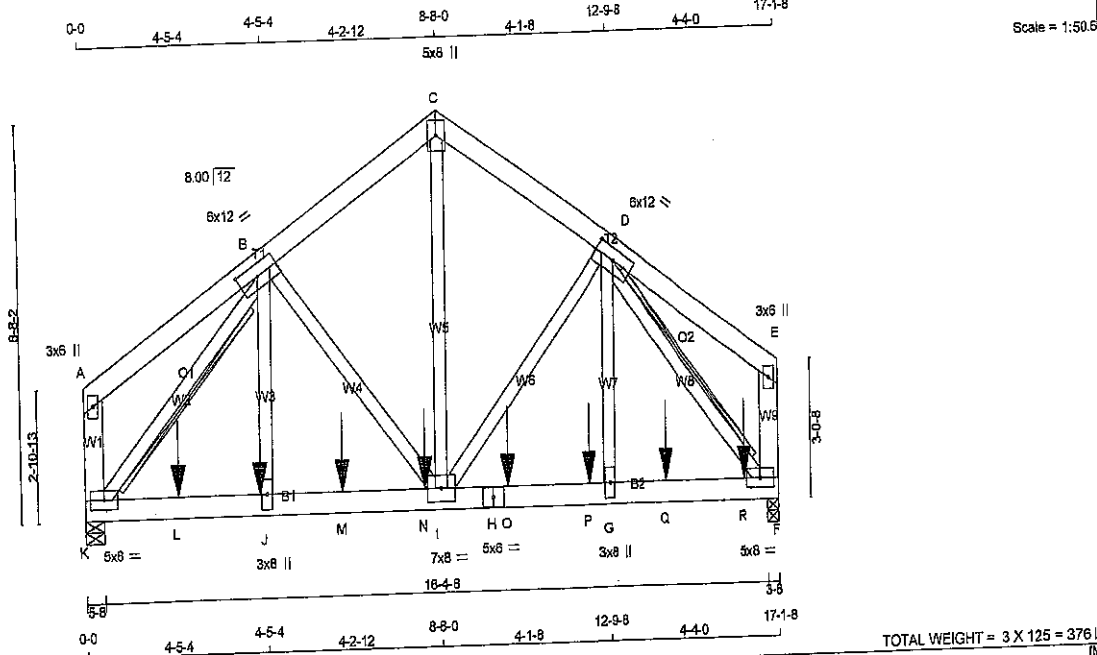
DWG NO. TAM 3339-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285748	T36A	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 3 X 125 = 375 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF	
C - E	2x6	DRY	No.2	SPF	
K - A	2x6	DRY	No.2	SPF	
F - E	2x6	DRY	No.2	SPF	
K - H	2x6	DRY	2100F 1.8E	SPF	
H - F	2x6	DRY	2100F 1.8E	SPF	

ALL WEBS 2x4 DRY No.2 SPF

EXCEPT

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	2 12	TOP
C - E	2 12	TOP
K - A	2 12	TOP
F - E	2 12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
K - H	2 9	SIDE(596.8)
H - F	2 8	SIDE(447.1)
WEBS : (0.122"x3") SPIRAL NAILS		
2x4	1 8	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	6.0			
B	TMVWW-I	MT20	6.0	12.0	3.00	5.75	
C	TTW+p	MT20	5.0	8.0			
D	TMVWW-I	MT20	6.0	12.0	3.00	5.75	
E	TMV+p	MT20	3.0	6.0			
F	BMVWW-I	MT20	5.0	8.0			
G	BMVWW	MT20	3.0	6.0			
H	BS-I	MT20	5.0	8.0			
I	BMVWW-I	MT20	7.0	8.0			
J	BMVWW	MT20	3.0	6.0			
K	BMVWW-I	MT20	5.0	8.0			

HANGERS NOTES

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	DOWN	UPLIFT	IN-SX
JT	VERT	HORZ	BRG	BRG
K	7363	0	5-8	5-8
F	8465	0	3-8	3-8

UNFACTORED REACTIONS	1ST CASE	MAX./MIN. COMPONENT REACTIONS	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED	SNOW					
K	5985	3438 / 0	1294 / 0	0 / 0	0 / 0	1298 / 0	0 / 0
F	6989	3924 / 0	1503 / 0	0 / 0	0 / 0	1462 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.84 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x8 DRY SPF No.2 T-BRACE AT B-K, D-F

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

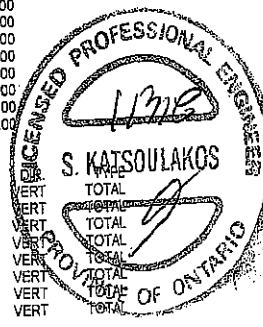
LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX.	MAX.	MEMB.	MAX.	FORCE	MAX.
MEMB.	FORCE	(LBS)	(PLF)	(LBS)	(LBS)	(LBS)	LENGTH	(LBS)	(LBS)	(LBS)
FR-TO										
A-B	-17 / 0	-84.3	-84.3	0.06 (1)	6.25	J-B	0 / 4120	0.22 (1)		
B-C	-5491 / 0	-84.3	-84.3	0.07 (1)	5.84	B-I	-1292 / 0	0.25 (1)		
C-D	-5489 / 0	-84.3	-84.3	0.07 (1)	5.84	I-C	0 / 5699	0.31 (1)		
D-E	-18 / 0	-84.3	-84.3	0.06 (1)	6.25	I-D	-1085 / 0	0.20 (1)		
K-A	-206 / 0	0.0	0.0	0.01 (1)	7.81	G-D	0 / 4021	0.22 (1)		
F-E	-205 / 0	0.0	0.0	0.01 (1)	7.81	K-B	-8403 / 0	0.47 (1)		
						D-F	-8344 / 0	0.47 (1)		
K-L	0 / 5340	-28.0	-28.0	0.27 (1)	10.00					
L-J	0 / 5340	-28.0	-28.0	0.27 (1)	10.00					
J-M	0 / 5392	-28.0	-28.0	0.25 (1)	10.00					
M-N	0 / 5392	-28.0	-28.0	0.25 (1)	10.00					
N-I	0 / 5234	-28.0	-28.0	0.25 (1)	10.00					
I-H	0 / 5234	-28.0	-28.0	0.25 (1)	10.00					
H-O	0 / 5234	-28.0	-28.0	0.25 (1)	10.00					
O-P	0 / 5234	-28.0	-28.0	0.25 (1)	10.00					
P-G	0 / 5234	-28.0	-28.0	0.25 (1)	10.00					
G-Q	0 / 5185	-28.0	-28.0	0.26 (1)	10.00					
Q-R	0 / 5185	-28.0	-28.0	0.26 (1)	10.00					
R-F	0 / 5185	-28.0	-28.0	0.26 (1)	10.00					

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE
J	4-3-4	-1826	-1826	BACK
L	2-3-4	-1712	-1712	BACK
M	6-3-4	-1826	-1826	BACK
N	8-3-4	-1712	-1712	BACK
O	10-3-4	-1712	-1712	BACK
P	12-3-4	-1712	-1712	BACK
Q	14-3-4	-1712	-1712	BACK
R	16-3-4	-1714	-1714	BACK



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.5 PSF
	DL = 3.0 PSF
BOT CH.	LL = 10.5 PSF
	DL = 7.0 PSF
TOTAL LOAD	= 46.1 PSF

SPACING = 240 IN. GIC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = 1/360 (0.57")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.05")
ALLOWABLE DEFL.(TL) = 1/250 (0.57")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.09")

CSI: TC=0.071/1.00 (B-C:1), BC=0.271/1.00 (J-K:1),
WB=0.471/1.00 (B-K:1), SS=0.501/1.00 (J-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.89 (D) (INPUT = 0.90)
JSI METAL= 0.64 (B) (INPUT = 1.00)

OWEN, TAN 3340-18
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285748	T36A	1	3	TRUSS DESC.	

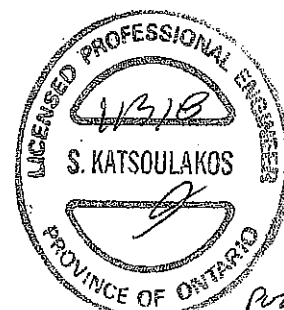
Tamarack Roof Truss, Burlington

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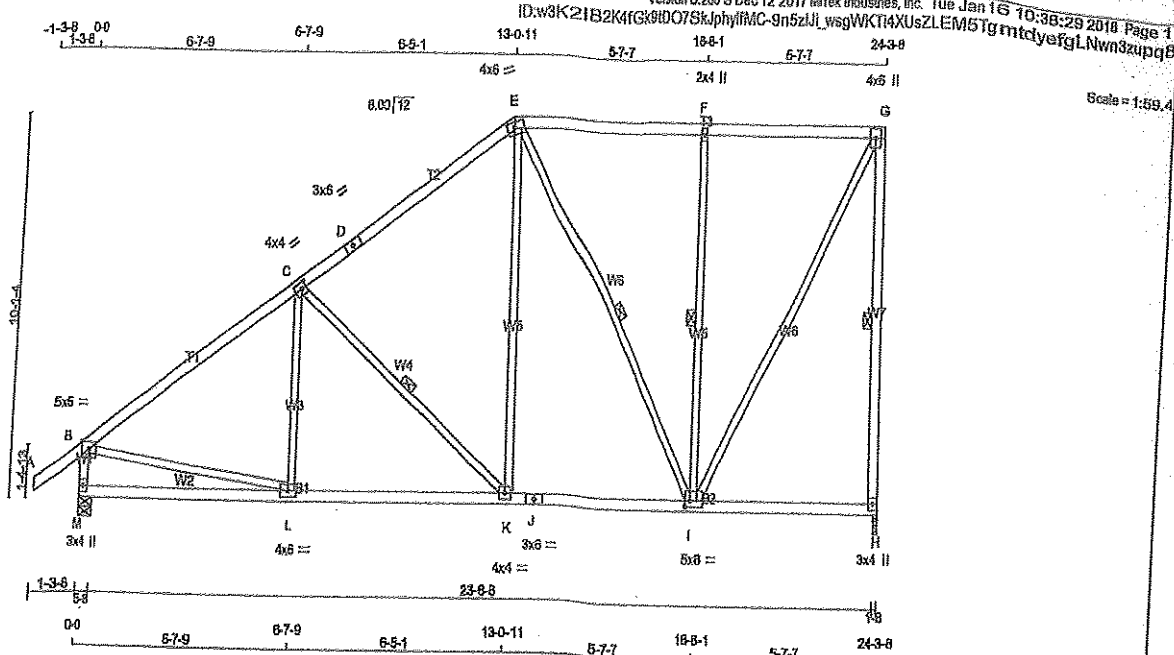
HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1711.9 lbs FACTORED DOWN AT 2-3-4, 1828.2 lbs FACTORED DOWN AT 4-3-4, 1828.2 lbs FACTORED DOWN AT 8-3-4, 1711.9 lbs FACTORED DOWN AT 8-3-4, 1711.9 lbs FACTORED DOWN AT 10-3-4, 1711.9 lbs FACTORED DOWN AT 12-3-4, AND 1711.9 lbs FACTORED DOWN AT 14-3-4, AND 1714.0 lbs FACTORED DOWN AT 16-3-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.



DWG NO. YAM 3340-18
STRUCTURAL
COMPONENT ONLY

JOB NAME 286039	TRUSS NAME T37	QUANTITY 2	PLY 1	JOB DESC. 43994	TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington						



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR
A - D	2x4 DRY	No.2	SPF
D - E	2x4 DRY	No.2	SPF
E - G	2x4 DRY	No.2	SPF
H - G	2x4 DRY	No.2	SPF
M - B	2x4 DRY	No.2	SPF
M - J	2x4 DRY	No.2	SPF
J - H	2x4 DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

CRY: SEASONED LUMBER

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	4.0	0.0	1.75	2.50
F	TMWW-w	MT20	2.0	4.0		
G	TMWW-p	MT20	4.0	0.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWWV-t	MT20	5.0	6.0	2.00	1.50
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	VERY	GROSS REACTION	GROSS REACTION	BRG	BRG
H	1353	0	1353	0	1-8
M	1479	0	1479	0	1-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
H	1119	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
M	1198	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.69 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-H, C-K, E-I, F-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX. FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 (LC)	MAX. (LC)	UNBRAC LENGTH FR-TO	MEMB.	FORCE (LBS)	MAX. (LBS)	CS1 (LC)
FR-TO		FROM	TO			FR-TO			
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	L-C	-1 / 267	0.08 (3)	
B-C	-1522 / 0	-84.3	-84.3	0.55 (1)	4.69	C-K	-594 / 0	0.30 (1)	
C-D	-1051 / 0	-84.3	-84.3	0.49 (1)	5.46	K-E	0 / 145	0.12 (2)	
D-E	-1051 / 0	-84.3	-84.3	0.49 (1)	5.46	E-I	-413 / 0	0.40 (1)	
E-F	-644 / 0	-84.3	-84.3	0.34 (1)	6.25	I-F	-566 / 0	0.41 (1)	
F-G	-644 / 0	-84.3	-84.3	0.34 (1)	6.25	L-G	0 / 1289	0.29 (1)	
H-G	-1289 / 0	0.0	0.0	0.85 (1)	5.66	B-L	0 / 1314	0.30 (1)	
M-B	-1404 / 0	0.0	0.0	0.14 (1)	5.87				
M-L	0 / 0	-28.0	-28.0	0.31 (3)	10.00				
L-K	0 / 1286	-28.0	-28.0	0.46 (2)	10.00				
K-J	0 / 848	-28.0	-28.0	0.31 (2)	10.00				
J-I	0 / 848	-28.0	-28.0	0.31 (2)	10.00				
I-H	0 / 0	-28.0	-28.0	0.21 (3)	10.00				

TOTAL WEIGHT = 2 X 122 = 243 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 240 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = 1/360 (0.81")
CALCULATED VERT. DEFL.(LL) = 1/899 (0.08")
ALLOWABLE DEFL.(TL) = 1/360 (0.81")
CALCULATED VERT. DEFL.(TL) = 1/899 (0.14")

CS1: TC=0.85/1.00 (G-H:1), BC=0.46/1.00 (K-L:2), WB=0.41/1.00 (F-I:1), SS1=0.23/1.00 (F-G:1)

COL LUMBER=1.00 NAIL=1.00 LG BEAD=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

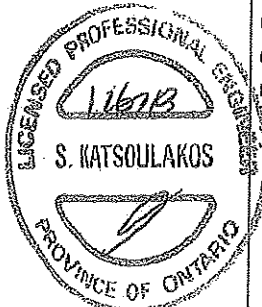
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 616 854 1687 822 2284 1656

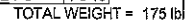
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

SSI GRIP= 0.83 (L) (INPUT = 0.90)
SSI METAL= 0.32 (L) (INPUT = 1.00)



DRWG NO. TAW 3894
STRUCTURAL

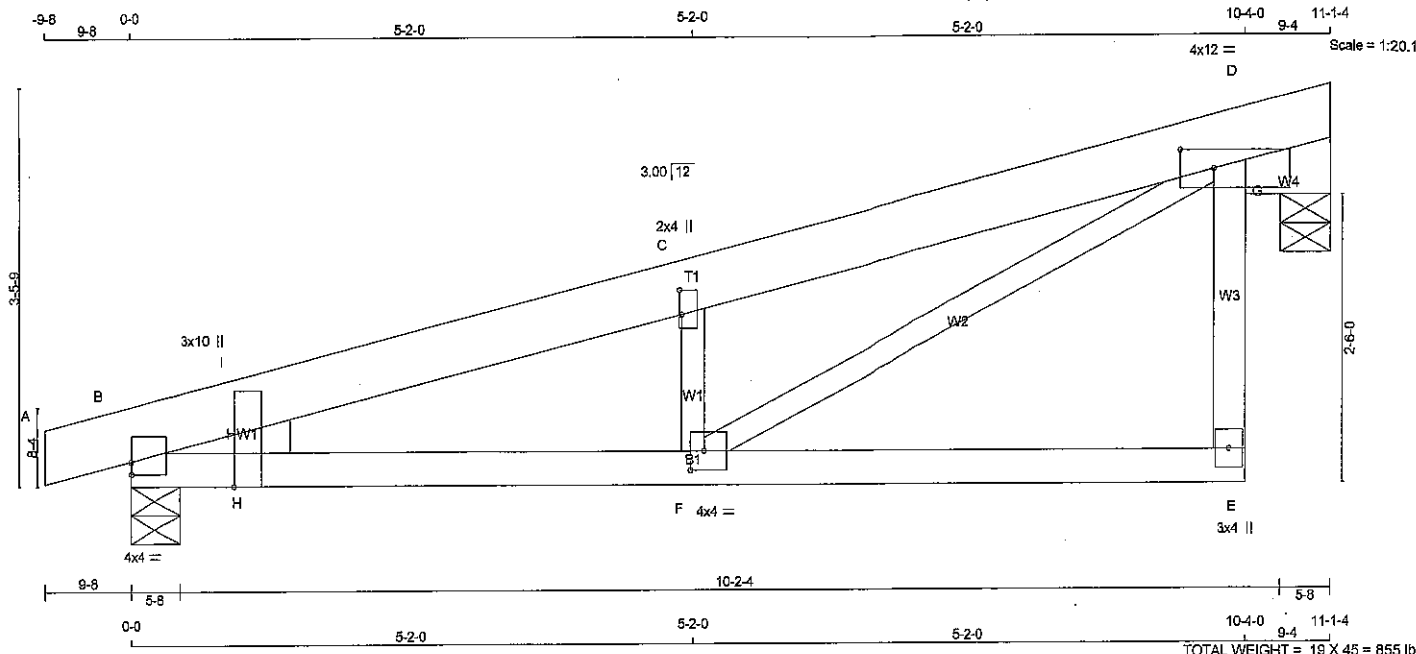


DWG NO. TAM 4488/17
STRUCTURAL
COMPONENT ONLY

JOB NAME 285781	TRUSS NAME T90TCX	QUANTITY 19	PLY 1	JOB DESC. 43954 TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 05 15:27:11 2017 Page 1
ID:w3K2IB2K4Gk9tD07SkJphylfMC-nlffB7ATX9kV4VBDPse5mggaZEvhjDzEfFIJygfFk



TOTAL WEIGHT = 19 X 45 = 855 lb [M]

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - D	2x8	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS 2x3 DRY No.2 SPF				
DRY; SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	4.0	4.0	1.25
B	WP+I	MT20	3.0	10.0	Edge 11.50
C	TMW+V	MT20	2.0	4.0	2.50 0.25
D	TMV+V	MT20	4.0	12.0	2.00 3.75
E	BMV+V	MT20	3.0	4.0	
F	BMV+V	MT20	4.0	4.0	2.00 1.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
JT	VERT	GROSS REACTION	HORZ	DOWN	HORZ	BRG	IN-SX	IN-SX	HEEL
J(D)	551	0	551	0	0	5-8	5-8	5-8	2x4 L
B	680	0	680	0	0	5-8	5-8	5-8	

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN. COMPONENT REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL		
J(D)	452	251 / 0	103 / 0	0 / 0	0 / 0	98 / 0	0 / 0		
B	548	321 / 0	114 / 0	0 / 0	0 / 0	114 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J(D), B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	-4 / 0	-84.3 -84.3	0.02 (1)	F-C	-420 / 0	0.08 (1)	
B-I	-1258 / 0	-84.3 -84.3	0.03 (1)	F-D	0 / 1157	0.26 (1)	
I-C	-1158 / 0	-84.3 -84.3	0.07 (1)	H-I	0 / 131	0.00 (1)	
C-D	-1152 / 0	-84.3 -84.3	0.17 (1)	D-J	-785 / 0	0.03 (1)	
E-G	0 / 108	0.0 0.0	0.34 (1)	G-J	0 / 558	0.00 (1)	
G-D	0 / 108	0.0 0.0	0.34 (1)				
B-H	0 / 1128	-28.0 -28.0	0.20 (1)				
H-F	0 / 1128	-28.0 -28.0	0.28 (2)				
F-E	0 / 103	-28.0 -28.0	0.17 (2)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.34")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.05")
ALLOWABLE DEFL.(TL) = $L/360$ (0.34")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.09")

CSI: TC=0.34 (E-G:1), BC=0.28 (F-H:2), WB=0.26 (D-F:1), SSI=0.15 (D-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX MIN
MT20	618	354	1567 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (F) (INPUT = 0.90)
JSI METAL= 0.40 (B) (INPUT = 1.00)

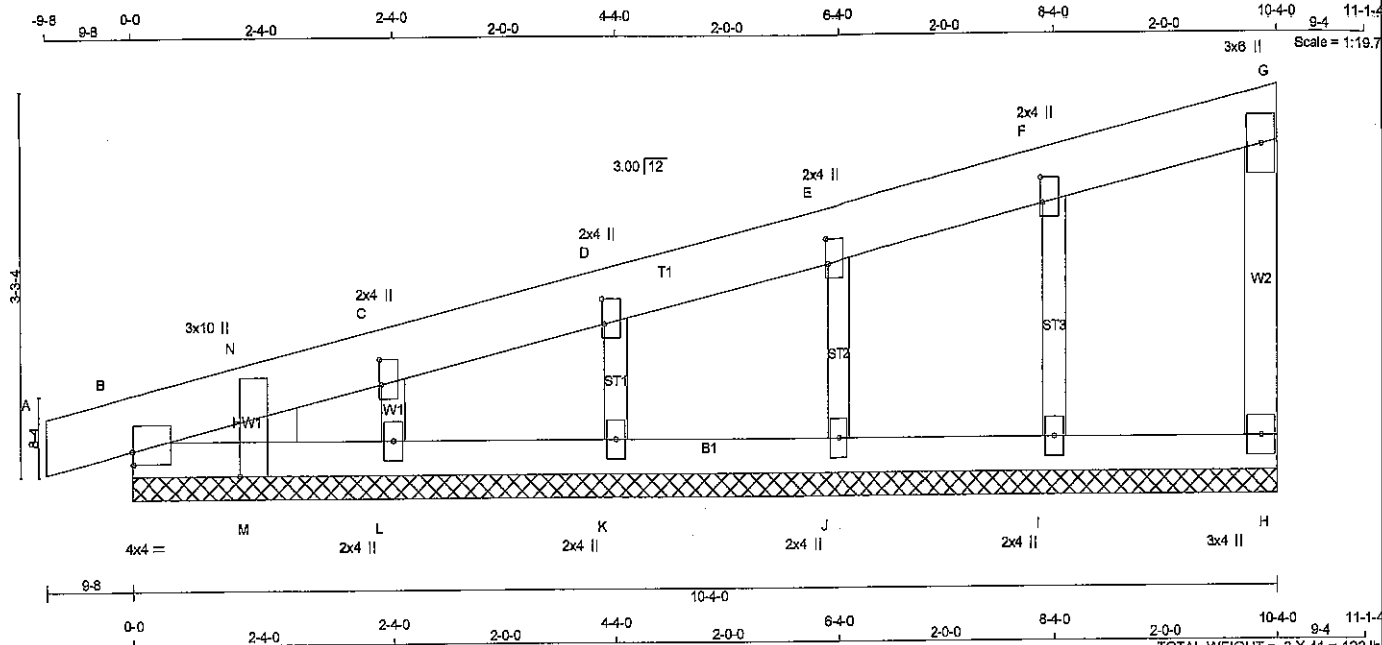


DWG NO. TAM 44882-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43854	DRWG NO.
285781	G90	3	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Tue Sep 05 15:26:58 2017 Page 1
ID:w3K2IB2K4fGk9DO7SkJphyfMC-fC0kw1zETu?gVb?x_gpdMjXsLB3yt5l_j03LaygfFx



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.	SPF
A - G	2x6	DRY	No.2	SPF		
H - G	2x4	DRY	No.2	SPF		
B - H	2x4	DRY	No.2	SPF		
ALL WEBS	2x3	DRY	No.2	SPF		
ALL GABLE WEBS	2x3	DRY	No.2	SPF		
DRY: SEASONED LUMBER.						

GABLE STUDS SPACED AT 2'-0" OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	4.0	4.0	1.25	
B	WP+I	MT20	3.0	10.0	Edge	11.50
C, D, E, F						
C	TMW+w	MT20	2.0	4.0	2.50	0.25
G	TMV+p	MT20	3.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I, J, K, L						
I	BMW1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

HEEL
WEDGE
2x4 L
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX LC1 (LC)	
FR-TO		FROM TO		FR-TO			
A-B	-41.0	-84.3	-84.3 0.02 (1)	10.00	I-F	-186.0	0.03 (1)
B-N	-46.0	-84.3	-84.3 0.02 (1)	6.25	J-E	-168.0	0.03 (1)
N-C	-14.3	-84.3	-84.3 0.03 (1)	6.25	K-D	-158.0	0.02 (1)
C-D	-18.0	-84.3	-84.3 0.03 (1)	6.25	L-C	-201.0	0.03 (1)
D-E	-9.0	-84.3	-84.3 0.02 (1)	10.00	M-N	0.41	0.00 (1)
E-F	-4.0	-84.3	-84.3 0.02 (1)	10.00			
F-G	-4.0	-84.3	-84.3 0.02 (1)	10.00			
H-G	-72.0	0.0	0.0 0.01 (1)	7.81			
B-M	0.28	-28.0	-28.0 0.02 (1)	10.00			
M-L	0.28	-28.0	-28.0 0.03 (2)	10.00			
L-K	0.16	-28.0	-28.0 0.02 (2)	10.00			
K-J	0.09	-28.0	-28.0 0.02 (2)	10.00			
J-I	0.4	-28.0	-28.0 0.03 (2)	10.00			
I-H	0.0	-28.0	-28.0 0.03 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(5% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.03 (C-D:1), BC=0.03 (I-J:2), WB=0.03 (F-I:1), SS=0.07 (C-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618 354 1687 822 2284 1856	

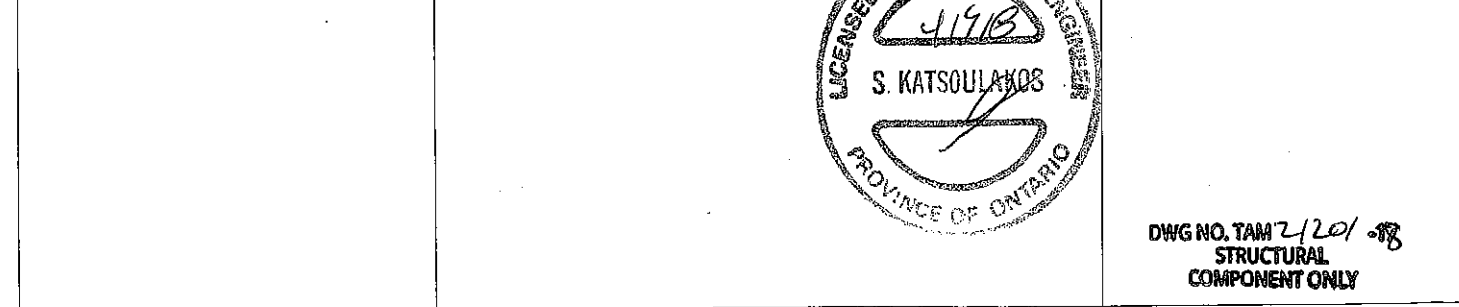
PLATE PLACEMENT TOL. = 0.250 inches

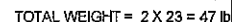
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (C) (INPUT = 0.90)
JSI METAL= 0.04 (L) (INPUT = 1.00)

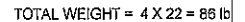


DWG NO. TAM 44892-17
STRUCTURAL
COMPONENT ONLY





DWG NO. TAM 24202-17
STRUCTURAL
COMPONENT ONLY

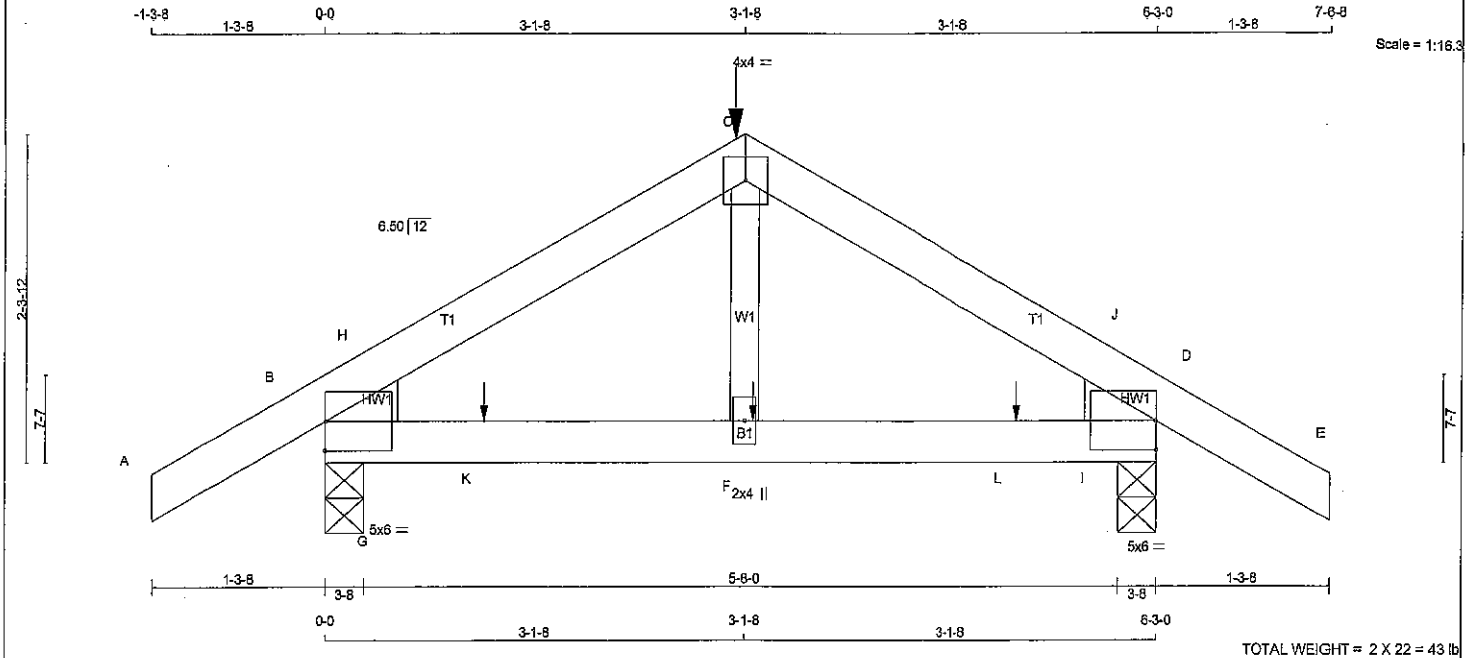


DWG NO. TAM 4483417
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
285781	T92Z	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 Mitek Industries, Inc. Tue Sep 05 15:27:12 2017 Page 1
ID:w3K2IB2K4fGk9DO7SkJphyfMC-Fus1s5CbmF0Mfhxw5Bjlvu_yAECRMcuPprmygfF



TOTAL WEIGHT = 2 X 22 = 43 lb

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMBH1-I	MT20	5.0	6.0
C	TTW-p	MT20	4.0	4.0
D	TMBH1-I	MT20	5.0	6.0
F	BMW-w	MT20	2.0	4.0

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 2.7 lbs FACTORED DOWN AT 3-1-8 ON TOP CHORD, AND AT 1-2-4, AND AT 3-2-4, AND AT 5-2-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG	HEEL
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	WEDGE
B	467	0	467	0	3-8	2x4 L
D	467	0	467	0	3-8	2x4 R

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	357	230 / 0	66 / 0	0 / 0	0 / 0	72 / 0	0 / 0
D	367	230 / 0	66 / 0	0 / 0	0 / 0	72 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	MEMB.	FORCE (LBS)	MAX	LC1
FR-TO		FROM	TO	FR-TO			
A-B	0 / 17	-84.3	-84.3 0.12 (1)	10.00	F-C	0 / 157	0.04 (2)
B-H	-399 / 0	-84.3	-84.3 0.06 (1)	6.25	G-H	-28 / 108	0.00 (1)
H-C	-340 / 0	-84.3	-84.3 0.09 (1)	6.25	I-J	-28 / 108	0.00 (1)
C-J	-340 / 0	-84.3	-84.3 0.09 (1)	6.25			
J-D	-399 / 0	-84.3	-84.3 0.06 (1)	6.25			
D-E	0 / 17	-84.3	-84.3 0.12 (1)	10.00			
B-G	0 / 293	-28.0	-28.0 0.08 (1)	10.00			
G-K	0 / 293	-28.0	-28.0 0.10 (1)	10.00			
K-F	0 / 293	-28.0	-28.0 0.10 (1)	10.00			
F-L	0 / 293	-28.0	-28.0 0.10 (1)	10.00			
L-I	0 / 293	-28.0	-28.0 0.10 (1)	10.00			
I-D	0 / 293	-28.0	-28.0 0.08 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX	MAX*	FACE	DIR.	TYPE
C	3-1-8	-2	-2	---	BACK	VERT	TOTAL
F	3-2-4	---	---	---	BACK	VERT	TOTAL
K	1-2-4	---	---	---	BACK	VERT	TOTAL
L	5-2-4	---	---	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.I.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.21")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.21")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TO=0.12 (D-E:1), BC=0.10 (F-G:1), WB=0.04 (C-F:2), SSI=0.10 (C-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1657 822 2264 1655

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (D) (INPUT = 0.90)
JSI METAL= 0.10 (D) (INPUT = 1.00)

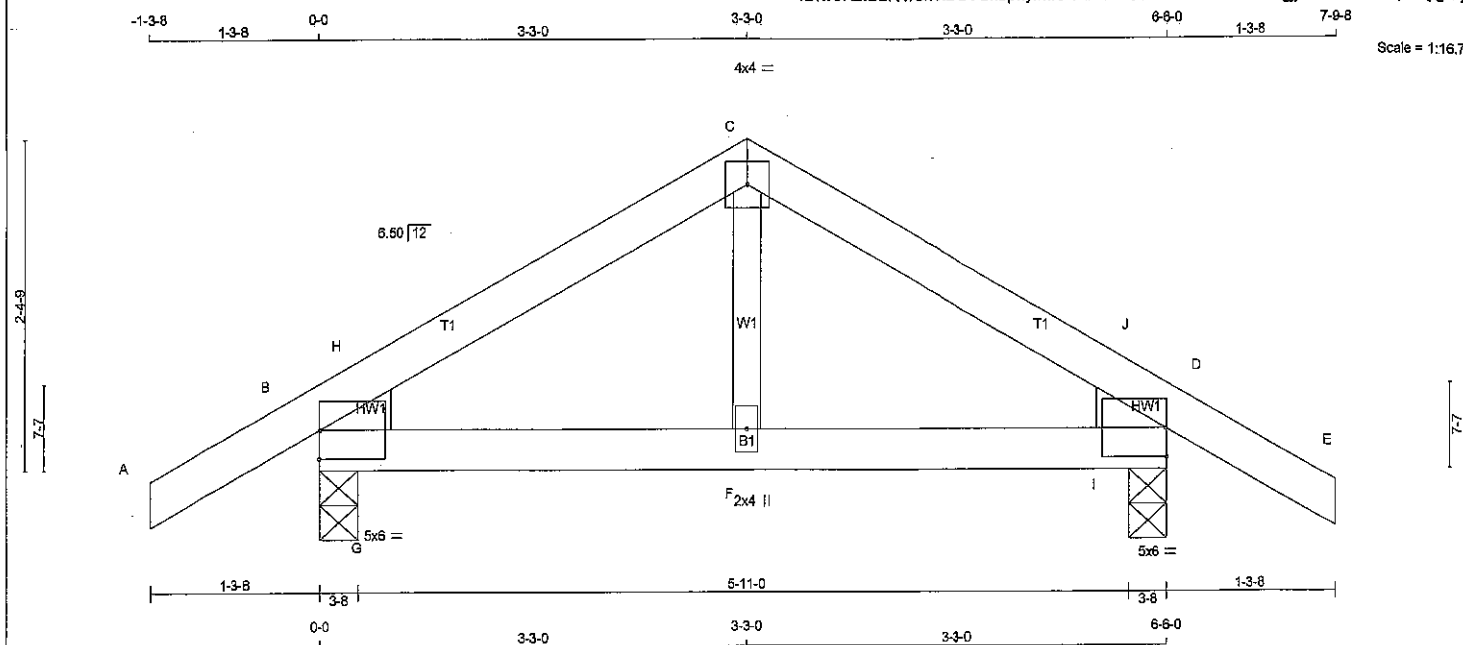


DRWG NO. TAM 4408517
STRUCTURAL
COMPONENT ONLY

JOB NAME 285781	TRUSS NAME T93	QUANTITY 2	PLY 1	JOB DESC. 43954	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.030 S Oct. 5 2016 Mittek Industries, Inc. Tue Sep 05 15:27:12 2017 Page 1
ID:w3K2iB2K4fGk9tDO7SkJphylfMC-Fus1s5Cbxmf0Mfthlw5BJlv3_yCECSMCuPprmygfF



TOTAL WEIGHT = 2 X 22 = 44 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
B - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY, SEASONED LUMBER.			

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMBH-1	MT20	5.0	6.0
C	TTW-p	MT20	4.0	4.0
D	TMBH-1	MT20	5.0	6.0
F	BMW+w	MT20	2.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			FACTORED			MAXIMUM FACTORED			INPUT			REQRD		
JT	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UP	IN-SX	IN-SX	BRG	IN-SX	IN-SX	HEEL	WEDGE	
B	480	0	480	0	0	0	3-8	3-8	2x4 L					
D	480	0	480	0	0	0	3-8	3-8	2x4 R					

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	377	235 / 0	68 / 0	0 / 0	0 / 0	73 / 0	0 / 0
D	377	235 / 0	68 / 0	0 / 0	0 / 0	73 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS			WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)
FR-TO		FROM TO	FR-TO		
A-B	0 / 17	-84.3 -84.3 0.11 (1)	F-C	0 / 185	0.04 (2)
B-H	-412 / 0	-84.3 -84.3 0.05 (1)	G-H	-34 / 113	0.00 (1)
H-C	-355 / 0	-84.3 -84.3 0.08 (1)	I-J	-34 / 113	0.00 (1)
C-J	-355 / 0	-84.3 -84.3 0.08 (1)			
J-D	-412 / 0	-84.3 -84.3 0.05 (1)			
D-E	0 / 17	-84.3 -84.3 0.11 (1)			
B-G	0 / 305	-28.0 -28.0 0.08 (1)			
G-F	0 / 305	-28.0 -28.0 0.10 (1)			
F-I	0 / 305	-28.0 -28.0 0.10 (1)			
I-D	0 / 305	-28.0 -28.0 0.08 (1)			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.8	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= 1/360 (0.22")
CALCULATED VERT. DEFL.(LL)= 1/999 (0.00")
ALLOWABLE DEFL.(TL)= 1/360 (0.22")
CALCULATED VERT. DEFL.(TL)= 1/999 (0.01")

CSI: TC=0.11 (D-E:1), BC=0.10 (F-G:1), WB=0.04 (C-F:2), SSI=0.09 (C-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (B) (INPUT = 0.90)
JSI METAL= 0.10 (D) (INPUT = 1.00)

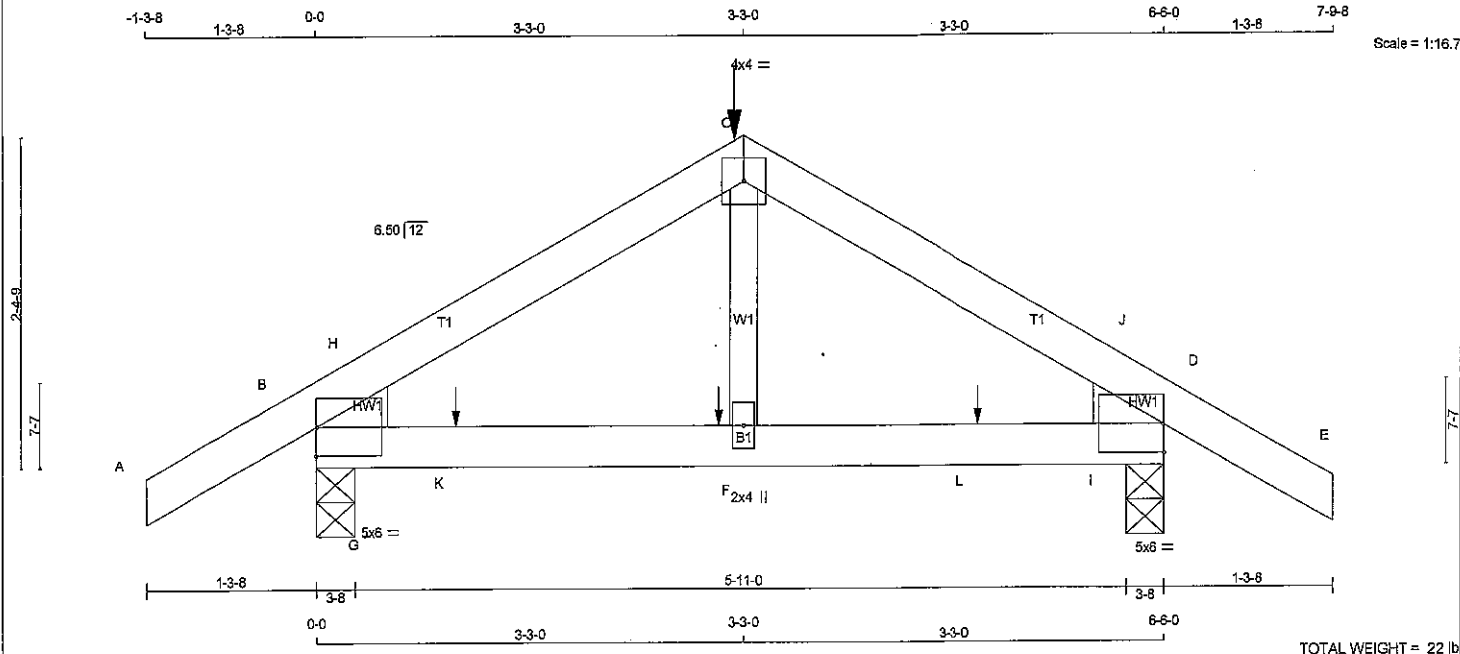


DWG NO. TAM 44866-17
STRUCTURAL
COMPONENT ONLY

JOB NAME 285781	TRUSS NAME T93Z	QUANTITY 1	PLY 1	JOB DESC. 43954 TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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LUMBER				DESCR.			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	SPF	SPF	SPF	SPF
A - C	2x4	DRY	No.2	SPF	SPF	SPF	SPF
C - E	2x4	DRY	No.2	SPF	SPF	SPF	SPF
B - D	2x4	DRY	No.2	SPF	SPF	SPF	SPF
ALL WEBS	2x3	DRY	No.2	SPF	SPF	SPF	SPF
DRY: SEASONED LUMBER.							

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN Y	X
B TMBH-1	MT20	5.0	6.0	
C TTW-p	MT20	4.0	4.0	
D TMBH-1	MT20	5.0	6.0	
F BMW-w	MT20	2.0	4.0	

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 2.5 lbs FACTORED DOWN AT 3-3-0 ON TOP CHORD, AND AT 1-0-12, AND AT 3-0-12, AND AT 5-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG		HEEL WEDGE	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX	2x4 L	2x4 R
B	481	0	481	0	0	3-8	3-8	3-8	3-8	2x4 L	2x4 R
D	481	0	481	0	0	3-8	3-8	3-8	3-8	2x4 L	2x4 R

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	378	236 / 0	68 / 0	0 / 0	0 / 0	74 / 0	0 / 0
D	378	236 / 0	68 / 0	0 / 0	0 / 0	74 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	FORCE (LBS)	MAX. LC1 (LC)	MAX. LC1 (LC)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 17	-84.3	-84.3 0.12 (1)	10.00	F-C	0 / 165	0.04 (2)
B-H	-414 / 0	-84.3	-84.3 0.07 (1)	6.25	G-H	-33 / 114	0.00 (1)
H-C	-357 / 0	-84.3	-84.3 0.10 (1)	6.25	I-J	-33 / 114	0.00 (1)
C-J	-357 / 0	-84.3	-84.3 0.10 (1)	6.25			
J-D	-414 / 0	-84.3	-84.3 0.07 (1)	6.25			
D-E	0 / 17	-84.3	-84.3 0.12 (1)	10.00			
B-G	0 / 307	-28.0	-28.0 0.09 (1)	10.00			
G-K	0 / 307	-28.0	-28.0 0.11 (1)	10.00			
K-F	0 / 307	-28.0	-28.0 0.11 (1)	10.00			
F-L	0 / 307	-28.0	-28.0 0.11 (1)	10.00			
L-I	0 / 307	-28.0	-28.0 0.11 (1)	10.00			
I-D	0 / 307	-28.0	-28.0 0.09 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	3-3-0	-2	-2	-	FRONT	VERT	TOTAL
K	3-0-12	-	-	-	FRONT	VERT	TOTAL
F	1-0-12	-	-	-	FRONT	VERT	TOTAL
L	5-0-12	-	-	-	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.22")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.22")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.01")

CSI: TC=0.12 (D-E:1), BC=0.11 (F-I:1), WB=0.04 (C-F:2), SSI=0.10 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

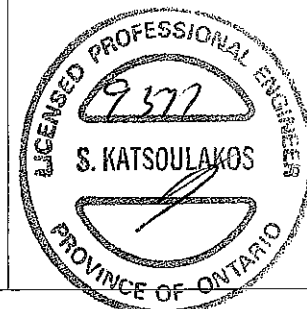
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (B) (INPUT = 0.80)
JSI METAL= 0.10 (D) (INPUT = 1.00)

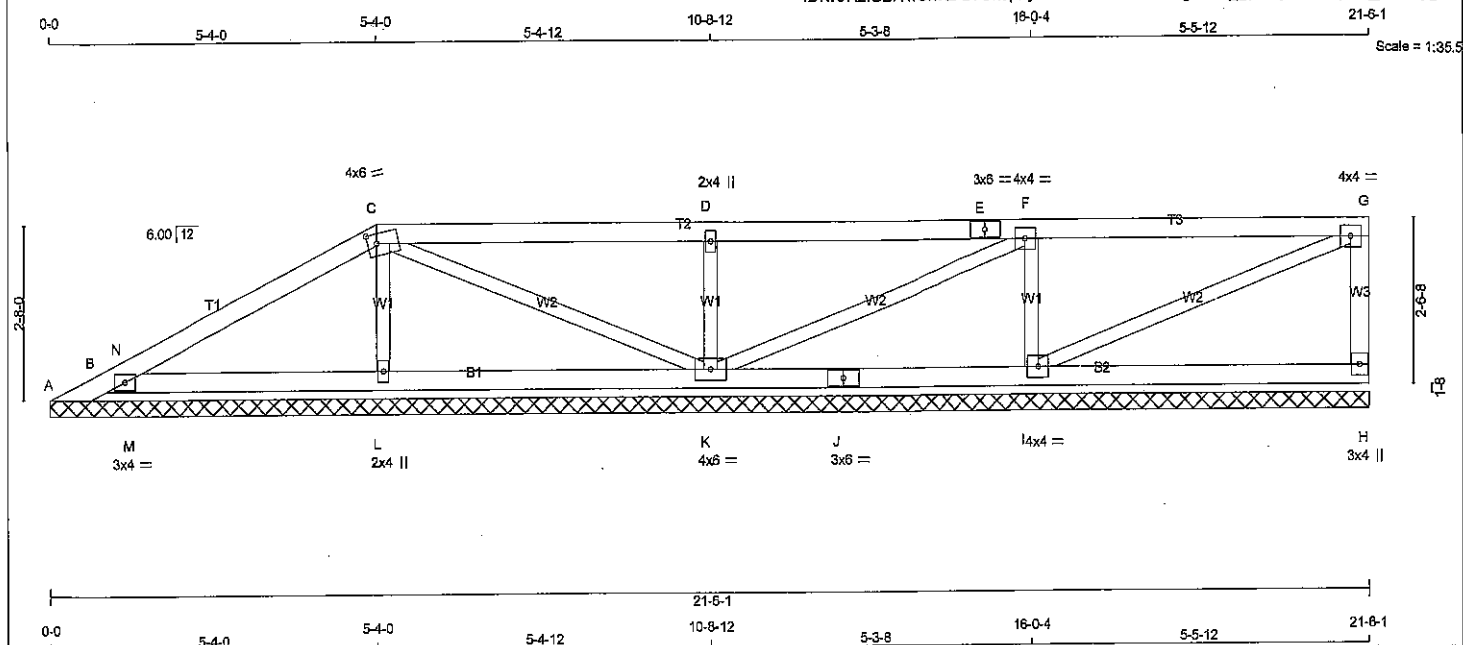


DWG NO. TAM 4488-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
285781	P1	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
H - G	2x4	DRY	No.2
B - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

BEARING NOTE: GAP BETWEEN INSIDE OF TOP CHORD BEARING AND FIRST DIAGONAL OR VERTICAL WEB SHALL NOT EXCEED 0.5 INCHES.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW-m	MT20	4.0	6.0	1.75	1.75
D	TMW+w	MT20	2.0	4.0		
E	TS-I	MT20	3.0	6.0		
F	TMVW-t	MT20	4.0	4.0		
G	TMVW-I	MT20	4.0	4.0		
H	BMV1+P	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	4.0		
J	BS-I	MT20	3.0	6.0		
K	BMVWW1-t	MT20	4.0	6.0		
L	BMV1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
A	-129 0	0 0	-129	21-6-1 (11-1120)S-1
H	250 0	250 0	0	21-6-1 (11-1120)S-1
B	508 0	508 0	0	21-6-1 (11-1120)S-1
L	446 0	446 0	0	21-6-1 (11-1120)S-1
I	658 0	658 0	0	21-6-1 (11-1120)S-1
K	667 0	667 0	0	21-6-1 (11-1120)S-1

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): H, L, I, K

PROVIDE ANCHORAGE AT BEARING JOINT A FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS
A	COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL
A	-104	0/-64 0/-23 0/0 0/0 0/-17 0/0
H	205	115/0 48/0 0/0 0/0 44/0 0/0
B	384	249/0 65/0 0/0 0/0 80/0 0/0
L	384	185/0 107/0 0/0 0/0 93/0 0/0
I	543	294/0 129/0 0/0 0/0 120/0 0/0
K	535	318/0 108/0 0/0 0/0 109/0 0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, H, B, L, I, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0/72	-101.8 -101.8	0.22 (1)	L-C	-284/0	0.05 (1)	
B-N	-125/0	-84.3 -84.3	0.21 (1)	I-G	0/10	0.00 (1)	
N-C	-59/0	-84.3 -84.3	0.17 (1)	C-K	-58/0	0.03 (1)	
C-D	0/22	-84.3 -84.3	0.33 (1)	I-F	-487/0	0.08 (1)	
D-E	0/21	-84.3 -84.3	0.34 (1)	K-D	-493/0	0.08 (1)	
E-F	0/21	-84.3 -84.3	0.34 (1)	K-F	-33/0	0.02 (1)	
F-G	-9/0	-84.3 -84.3	0.34 (1)	M-N	-58/149	0.00 (1)	
H-G	-190/0	0.0 0.0	0.02 (1)				
B-M	0/43	-28.0 -28.0	0.06 (1)				
M-L	0/43	-28.0 -28.0	0.15 (2)				
L-K	0/30	-28.0 -28.0	0.15 (2)				
K-J	0/9	-28.0 -28.0	0.21 (2)				
J-I	0/9	-28.0 -28.0	0.21 (2)				
I-H	0/0	-28.0 -28.0	0.21 (2)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.34 (D-F-1), BC=0.21 (I-K-2), WB=0.08 (D-K-1), SSI=0.22 (F-G-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (C) (INPUT = 0.90)
JSI METAL= 0.24 (E) (INPUT = 1.00)

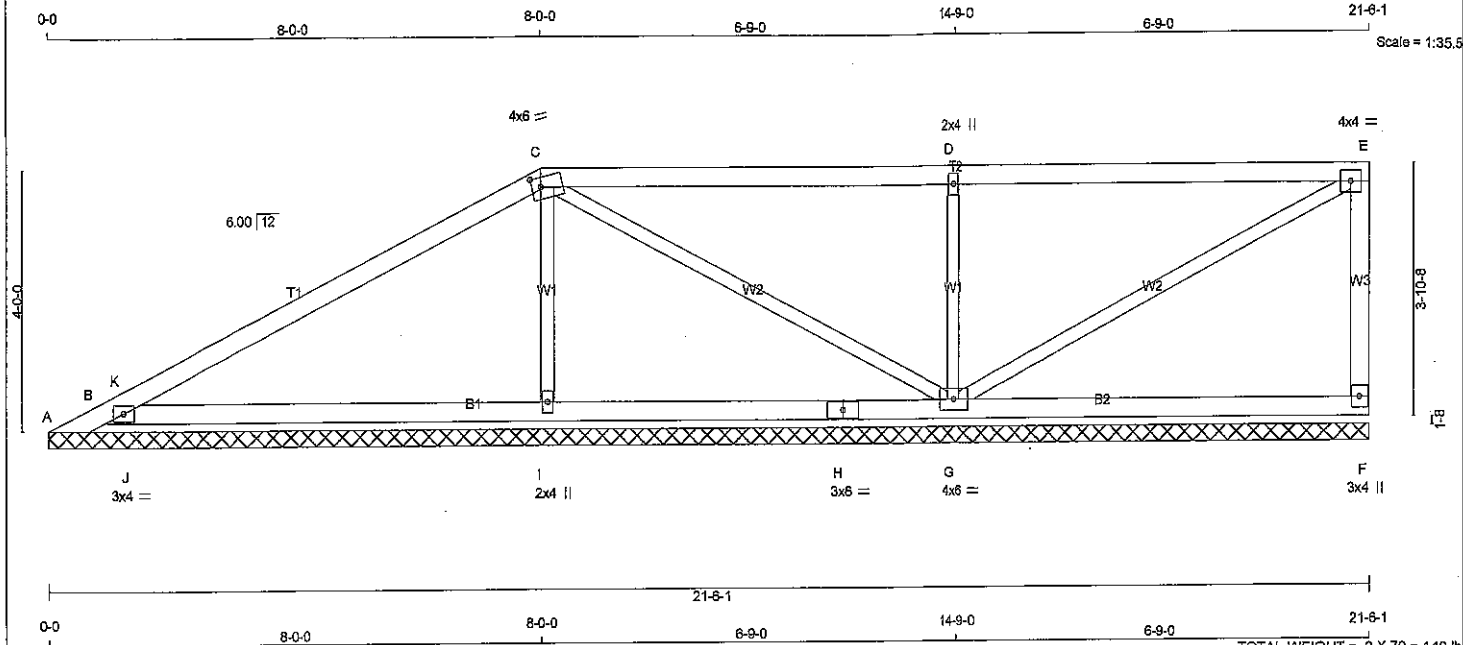


DRWG NO. TAM 4489417
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
285781	P2	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
F - E	2x4	DRY	No.2
B - H	2x4	DRY	No.2
H - F	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

BEARING NOTE: GAP BETWEEN INSIDE OF TOP CHORD BEARING AND FIRST DIAGONAL OR VERTICAL WEB SHALL NOT EXCEED 0.5 INCHES.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMB1-l	MT20	3.0	4.0
C	TTWW-m	MT20	4.0	6.0 1.75 1.75
D	TMWf-w	MT20	2.0	4.0
E	TMWf-t	MT20	4.0	4.0
F	BMV1+p	MT20	3.0	4.0
G	BMWW1-l	MT20	4.0	6.0
H	BS-t	MT20	3.0	6.0
I	BMW1+w	MT20	2.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
A	-490	0	0	0	-490
F	287	0	287	0	0
B	1065	0	1065	0	0
I	573	0	573	0	0
G	962	0	962	0	0

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): F, I, G

PROVIDE ANCHORAGE AT BEARING JOINT A FOR 490 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
A	-385	0/-235	0/-83	0/0	0/0	0/-77	0/0
F	239	128/0	57/0	0/0	0/0	53/0	0/0
B	845	513/0	160/0	0/0	0/0	173/0	0/0
I	495	236/0	139/0	0/0	0/0	120/0	0/0
G	774	456/0	159/0	0/0	0/0	159/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, F, B, I, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0/222	-101.8 -101.8	0.67 (1)	I-C	-351/0	0.08 (1)	
B-K	-318/0	-84.3 -84.3	0.64 (1)	C-G	-89/0	0.10 (1)	
K-C	-111/0	-84.3 -84.3	0.45 (1)	G-D	-711/0	0.17 (1)	
C-D	-2/10	-84.3 -84.3	0.65 (1)	D-E	-11/2	0.01 (1)	
D-E	-2/10	-84.3 -84.3	0.65 (1)	E-F	0/493	0.00 (1)	
E-F	-2/10/0	0.0 0.0	0.05 (1)				
B-J	0/79	-28.0 -28.0	0.18 (3)				
J-I	0/79	-28.0 -28.0	0.27 (2)				
I-H	0/69	-28.0 -28.0	0.31 (3)				
H-G	0/69	-28.0 -28.0	0.31 (3)				
G-F	0/0	-28.0 -28.0	0.31 (3)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 25.6	PSF
	DL	= 3.0	PSF
BOT CH.	LL	= 10.5	PSF
	DL	= 7.0	PSF
TOTAL LOAD	=	48.1	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.67 (A-B:1), BC=0.31 (G-I:3), WB=0.17 (D-G:1), SSI=0.41 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

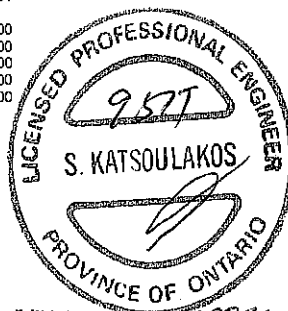
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618 354 1687 822 2284 1658	

PLATE PLACEMENT-TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

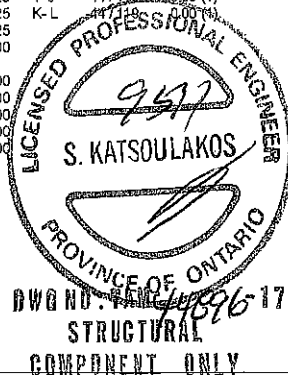
JSI GRIP= 0.81 (B) (INPUT = 0.90)
JSI METAL= 0.21 (B) (INPUT = 1.00)



DWG NO. TMR 44875-17
STRUCTURAL
COMPONENT ONLY



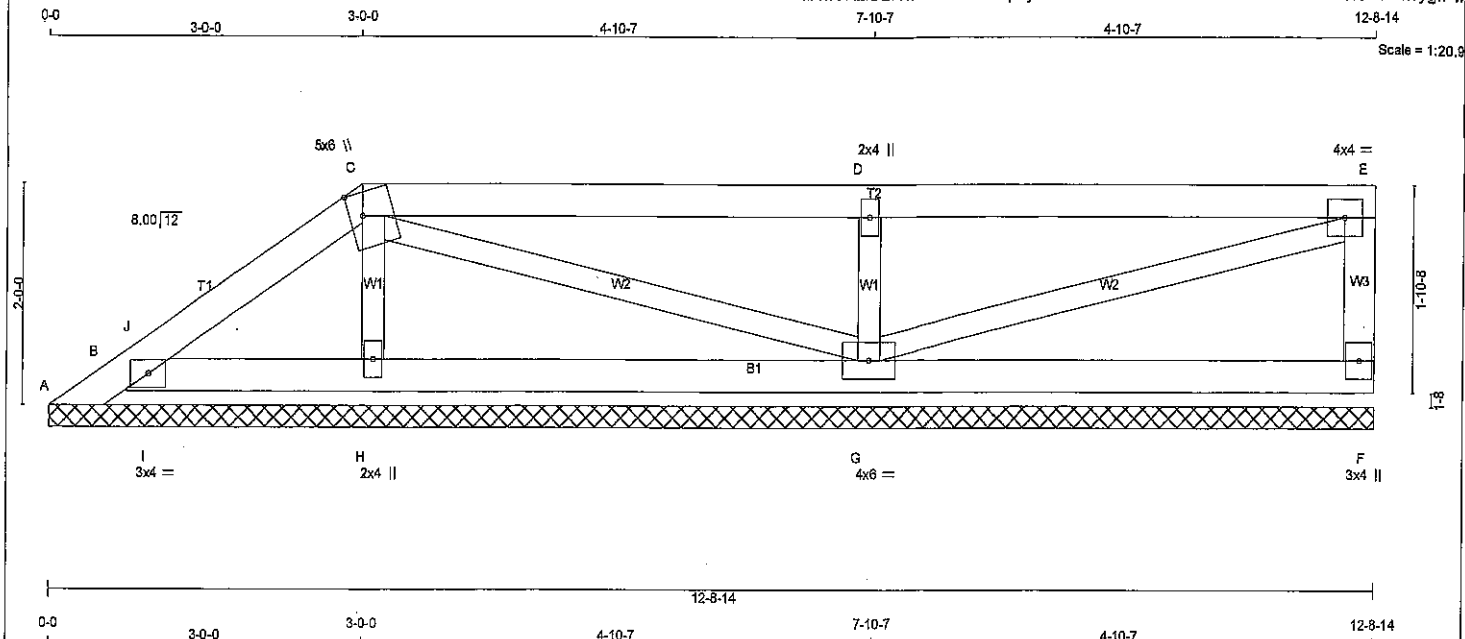
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JSI METAL= 0.06 (E) (INPUT = 1.00)



JOB NAME 285781	TRUSS NAME P6	QUANTITY 2	PLY 1	JOB DESC. 43954 TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 Mitek Industries, Inc. Tue Sep 05 15:26:59 2017 Page 1
ID:w3K2IB2K4fGk9IDO7SkJphyfMC-7Oa78e1b?n0si9BUIB29ZGdkkV0hJfSDNldt0ygfFw



LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

BEARING NOTE: GAP BETWEEN INSIDE OF TOP CHORD BEARING AND FIRST DIAGONAL OR VERTICAL WEB SHALL NOT EXCEED 0.5 INCHES.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMB1-I	MT20	3.0	4.0
C	TTWW+m	MT20	5.0	6.0 2.50 1.50
D	TMW+w	MT20	2.0	4.0
E	TMW+V-t	MT20	4.0	4.0
F	BMV1+p	MT20	3.0	4.0
G	BMVWW1-t	MT20	4.0	6.0
H	BMV1+w	MT20	2.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	RECD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
A	-12	0	3	0	-12	12-8-14 (11-1128)4
F	208	0	208	0	0	12-8-14 (11-1128)4
B	228	0	228	0	0	12-8-14 (11-1128)4
H	311	0	311	0	0	12-8-14 (11-1128)4
G	677	0	677	0	0	12-8-14 (11-1128)4

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): F, H, G

PROVIDE ANCHORAGE AT BEARING JOINT A FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
A	-9	0/-9	0/-1	0/0	0/0	2/0	0/0
F	173	94/0	40/0	0/0	0/0	38/0	0/0
B	168	122/0	18/0	0/0	0/0	30/0	0/0
H	271	125/0	78/0	0/0	0/0	67/0	0/0
G	550	315/0	118/0	0/0	0/0	116/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, F, B, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO				
A-B	0/23	-101.8 -101.8 0.05 (1)	10.00	H-C -196/0 0.03 (1)
B-J	-88/0	-84.3 -84.3 0.04 (1)	6.25	C-G -23/0 0.01 (1)
J-C	-45/0	-84.3 -84.3 0.04 (1)	6.25	G-D -512/0 0.07 (1)
C-D	0/2	-84.3 -84.3 0.34 (1)	10.00	G-E -2/0 0.00 (1)
D-E	0/2	-84.3 -84.3 0.34 (1)	10.00	I-J -52/6 0.00 (1)
F-E	-155/0	0.0 0.0 0.02 (1)	7.81	
B-I	0/33	-28.0 -28.0 0.03 (1)	10.00	
I-H	0/33	-28.0 -28.0 0.08 (2)	10.00	
H-G	0/20	-28.0 -28.0 0.17 (2)	10.00	
G-F	0/0	-28.0 -28.0 0.17 (3)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.34 (C-D:1), BC=0.17 (G-H:2), WB=0.07 (D-G:1), SSI=0.20 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

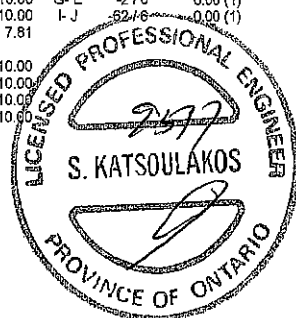
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 B22 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (D) (INPUT = 0.90)
JSI METAL= 0.09 (D) (INPUT = 1.00)



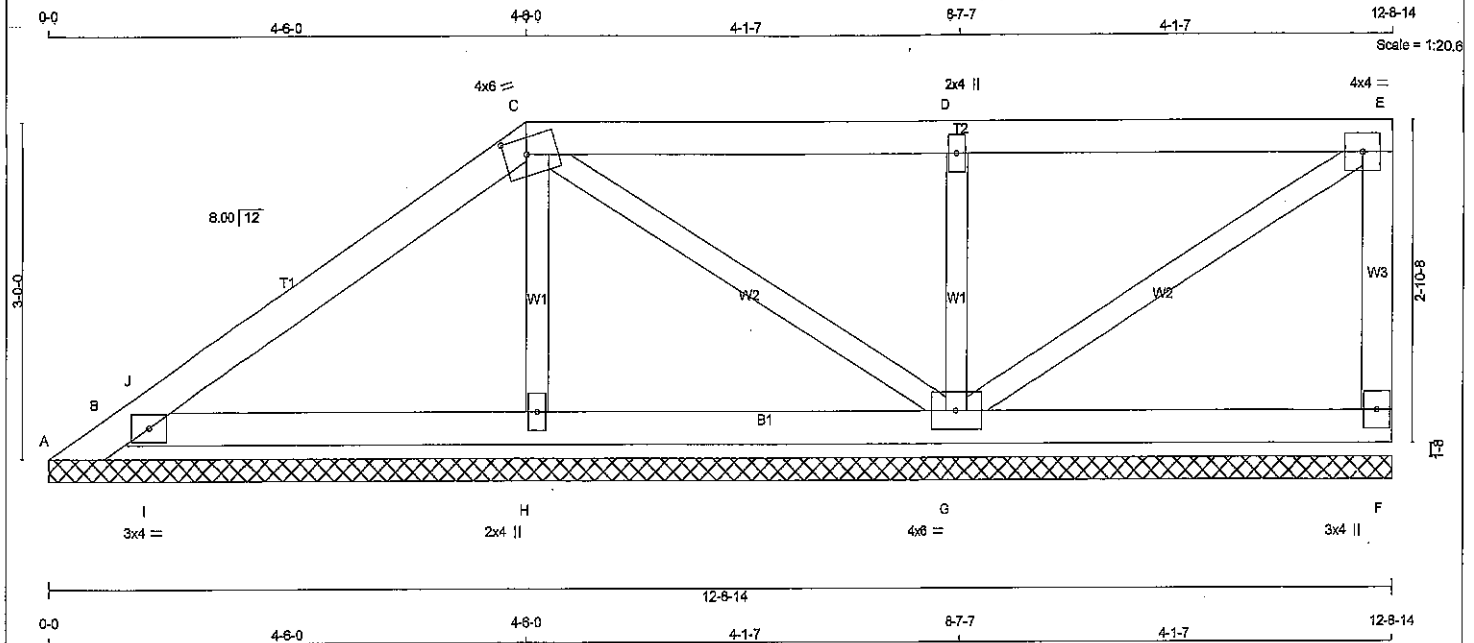
DRWG NO. TAM 4489717
STRUCTURAL
COMPONENT ONLY

JOB NAME 285781	TRUSS NAME P7	QUANTITY 2	PLY 1	JOB DESC. 43954 TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:w3K2IB2K4fGk9ID07SkJphylfMC-ba7VL_2Dm48jwpkN2PiHInpq?8s5QmvrR1VAQSYgfFv



LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

BEARING NOTE: GAP BETWEEN INSIDE OF TOP CHORD BEARING AND FIRST DIAGONAL OR VERTICAL WEB SHALL NOT EXCEED 0.5 INCHES.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW-m	MT20	4.0	6.0	1.75	2.50
D	TMVW-w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	4.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMVWW1-t	MT20	4.0	6.0		
H	BMV1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ
A	-142	0	0	-142
F	174	0	174	0
B	474	0	474	0
H	313	0	313	0
G	596	0	596	0

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): F, H, G

PROVIDE ANCHORAGE AT BEARING JOINT A FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
	COMBINED						
A	-118	0/-67	0/-27	0/0	0/0	0/-22	0/0
F	145	77/0	35/0	0/0	0/0	32/0	0/0
B	373	230/0	67/0	0/0	0/0	76/0	0/0
H	273	128/0	79/0	0/0	0/0	68/0	0/0
G	479	282/0	98/0	0/0	0/0	98/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, F, B, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0/89	-101.8 -101.8	0.17 (1)	10.00	H-C	-189/0	0.03 (1)
B-J	-151/0	-84.3 -84.3	0.16 (1)	6.25	C-G	-58/0	0.02 (1)
J-C	-69/0	-84.3 -84.3	0.12 (1)	6.25	G-D	-433/0	0.07 (1)
C-D	0/9	-84.3 -84.3	0.24 (1)	10.00	G-E	-11/0	0.00 (1)
D-E	0/9	-84.3 -84.3	0.24 (1)	10.00	I-J	-26/173	0.09 (4)
F-E	-127/0	0.0 0.0	0.02 (1)	7.81			
B-I	0/47	-28.0 -28.0	0.04 (3)	10.00			
I-H	0/47	-28.0 -28.0	0.09 (2)	10.00			
H-G	0/40	-28.0 -28.0	0.12 (2)	10.00			
G-F	0/0	-28.0 -28.0	0.12 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.24 (C-D:1), BC=0.12 (G-H:2), WB=0.07 (D-G:1), SSI=0.17 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

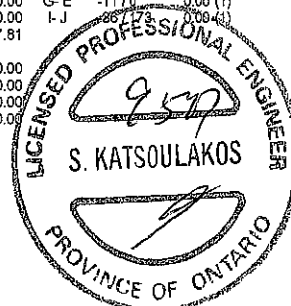
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	616	354	1667
	622	2284	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.35 (B) (INPUT = 0.90)
JSI METAL= 0.08 (D) (INPUT = 1.00)



DRWG NO. TAM 4487817
STRUCTURAL
COMPONENT ONLY

LUS - Double Shear Joist Hangers



All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

MATERIAL: 18 gauge

FINISH: G90 galvanized

DESIGN:

- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

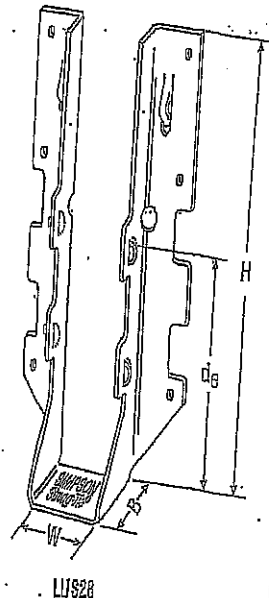
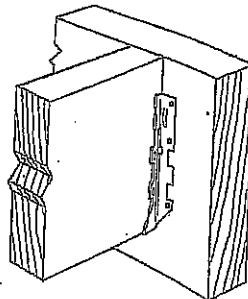
INSTALLATION:

- Use all specified fasteners
- Nails: 16d = 0.132" dia. x 3 1/2" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

- These hangers cannot be modified.

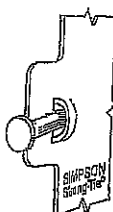
Typical LUS Installation



LUS28

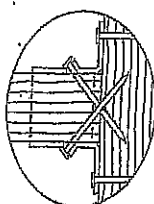
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
		W	H	B	d _g ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _u =1.15)	Normal (K _n =1.00)	Uplift (K _u =1.15)	Normal (K _n =1.00)
LUS24	18	1 1/8	3 1/4	1 1/4	1 1/8	4-10d	2-10d	710	1880	845	1156
LUS24-2	18	3/4	8 1/4	2	1 1/8	4-16d	2-16d	835	2020	590	1435
LUS26	18	1 1/8	4 1/4	1 1/4	3/8	4-10d	4-10d	1420	2170	1290	1830
LUS26-2	18	3/4	4 1/4	2	3/4	4-16d	4-16d	1720	2595	1545	1920
LUS26-3	18	4/4	4 1/4	2	3/4	4-16d	4-16d	1720	2595	1545	2840
LUS28	18	1 1/8	6 1/4	1 1/4	3/8	6-10d	4-10d	1420	2520	1290	1790
LUS28-2	18	3/4	7	2	3/4	6-16d	4-16d	1720	3325	1545	2575
LUS28-3	18	4/4	8 1/4	2	3/4	6-16d	4-16d	1720	3325	1545	2375
LUS210	18	1 1/8	7 1/4	1 1/4	3/8	8-10d	4-10d	1420	2785	1290	2210
LUS210-2	18	3/4	9	2	3/4	8-16d	6-16d	2590	4500	2320	3195
LUS210-3	18	4/4	8 1/4	2	3/4	8-16d	6-16d	2590	4345	2320	2375

1. d_g is the distance from the seat of the hanger to the highest joist nail.

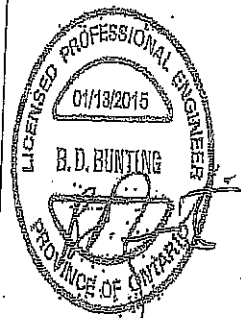


Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,530



Double Shear Nailing Top View.



HUS/LJS - Double Shear Joist Hangers



All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: See table

FINISH: G90 galvanized

DESIGN:

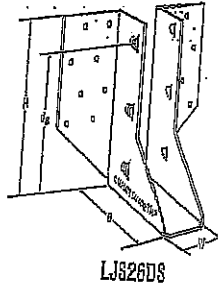
- Factored resistances are in accordance with CSA 086-14
- Uplift resistances have been increased 15%. No further increase is permitted
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

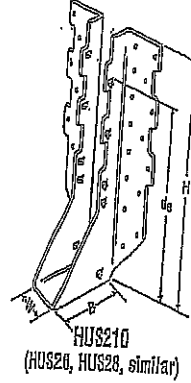
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

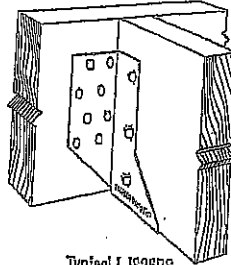
- See current catalogue for options



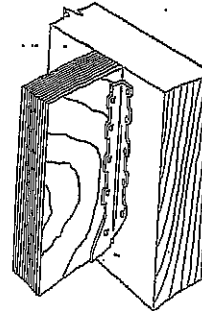
LJS26DS



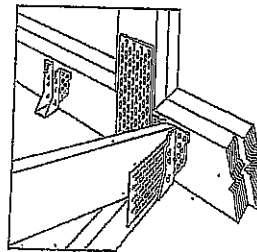
HUS210
(HUS20, HUS28, similar)



Typical LJS26DS
Installation



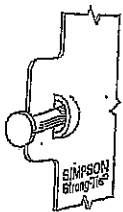
Typical HUS
Installation



Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

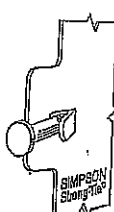
Model No.	Ga	Dimensions (in)			Fasteners			Factored Resistance (lbs)			
		W	H	B	dg ¹	Face	Joist	D-Fit-L		S-P-F	
								Uplift (K _u =1.15)	Normal (K _n =1.00)	Uplift (K _u =1.15)	Normal (K _n =1.00)
LJS26DS	18	1 9/16	5	3 1/2	4 3/4	16-16d	8-16d	2055	4285	1460	4115
HUS20	18	1 1/8	5 1/4	3	3 1/4	14-16d	8-16d	2705	4940	2025	3875
HUS28	18	1 1/8	7 1/4	3	6 1/4	22-16d	8-16d	3805	5365	2875	4945
HUS210	18	1 1/8	9 1/4	3	7 3/4	30-16d	10-16d	4505	5785	4015	4740
HUS1.81/10	18	1 1/8	9	3	8	30-16d	10-16d	4505	6450	4015	5200

1. d_g is the distance from the seat of the hanger to the highest joist nail.

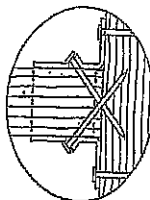


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

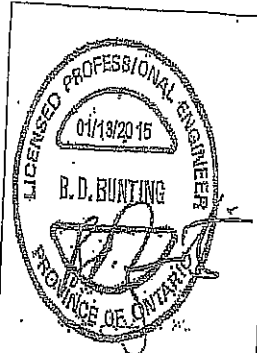
U.S. Patent
5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



DESIGN

1. All dimensions are in inches unless otherwise noted. 2. All dimensions are minimum unless otherwise noted. 3. All dimensions are maximum unless otherwise noted. 4. All dimensions are nominal unless otherwise noted. 5. All dimensions are actual unless otherwise noted. 6. All dimensions are nominal unless otherwise noted. 7. All dimensions are actual unless otherwise noted. 8. All dimensions are nominal unless otherwise noted. 9. All dimensions are actual unless otherwise noted. 10. All dimensions are nominal unless otherwise noted.

800-999-5099
www.simpsonstrongtie.com

HGUS - Double Shear Joist Hangers



All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: 12 gauge

FINISH: G90 galvanized

DESIGN:

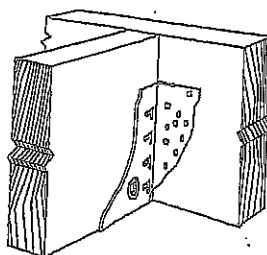
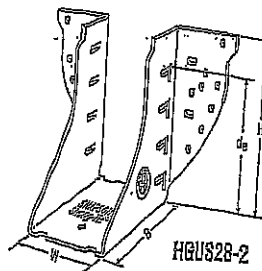
- Factored resistances are in accordance with CSA D86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

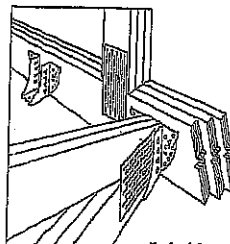
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3 1/4" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

- See current catalogue for options.



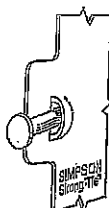
Typical HGUS Installation



Typical HGUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

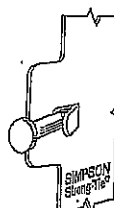
Model No.	Ga	Dimensions (in)					Fasteners		Factored Resistance (lbf)			
		W	H	B	d ₁	d ₂	Face	Joist	D.F.L		S-P-F	
									Uplift	Normal	Uplift	Normal
									(K _u =1.16)	(K _n =1.90)	(K _u =1.16)	(K _n =1.90)
HGUS28	12	1 1/2"	5 1/2"	5	4 1/4"	20-16d	8-16d	2885	6825	2885	5700	
HGUS28-2	12	3 1/2"	6 1/2"	4	4 1/4"	20-16d	8-16d	4385	8850	3100	8855	
HGUS20-3	12	4 1/2"	5 1/2"	4	4 1/4"	20-16d	8-16d	4385	8850	3100	8855	
HGUS28-4	12	6 1/2"	6 1/2"	4	4 1/4"	20-16d	8-16d	4385	8850	3100	8855	
HGUS28	12	1 1/2"	7 1/2"	6	6 1/4"	38-16d	12-16d	9310	7675	3100	8800	
HGUS28-2	12	3 1/2"	7 1/2"	4	6 1/4"	38-16d	12-16d	6070	12990	4310	9215	
HGUS28-3	12	4 1/2"	7 1/2"	4	6 1/4"	38-16d	12-16d	6070	12990	4310	9215	
HGUS28-4	12	6 1/2"	7 1/2"	4	6 1/4"	38-16d	12-16d	6070	12990	4310	9215	
HGUS210-2	12	3 1/2"	8 1/2"	4	8 1/4"	46-16d	16-16d	8840	14645	4855	10400	
HGUS210-3	12	4 1/2"	9 1/2"	4	8 1/4"	46-16d	16-16d	8840	14645	4855	10400	
HGUS210-4	12	6 1/2"	9 1/2"	4	8 1/4"	46-16d	16-16d	8840	14645	4855	10400	
HGUS212-4	12	6 1/2"	10 1/2"	4	10 1/4"	58-16d	20-16d	7840	14995	5425	10645	
HGUS214-4	12	6 1/2"	12 1/2"	4	11 1/4"	66-16d	22-16d	10130	16400	7195	11845	

1. d₂ is the distance from the seat of the hanger to the highest joist nail.

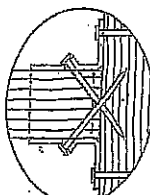


Double Shear Nailing prevents tabs breaking off (available on some models).

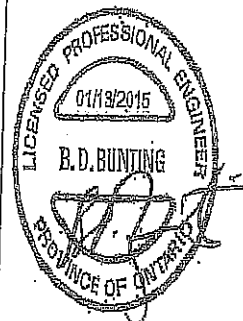
U.S. Patent
5,603,880



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



100-998-3899
www.simpsonstrongtie.com

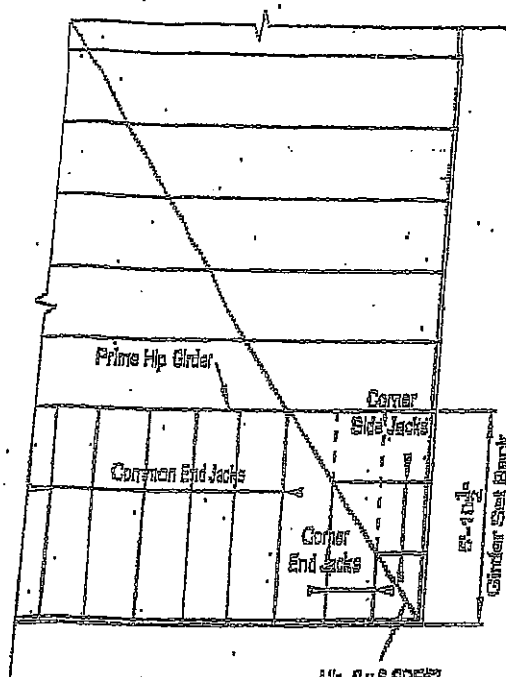
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100-998-3899
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MICRO CITY

ENGINEERING SERVICES INC.

TEL: (619) 287-2242



45° Hip End

LUMBER SPECIFICATION

TOP CHORD : 2x4 SPF#2

BOTTOM CHORD : 2x4 SPF#2

WEBS : 2x3 SPF#2

UNLESS OTHERWISE SHOWN

DESIGN LOAD:

TOP CHORD LIVE LOAD : 39.4 P.S.F.

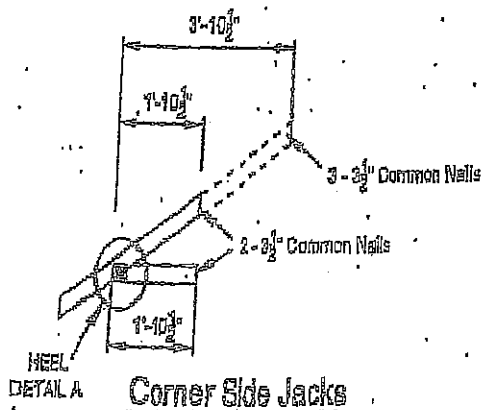
TOP CHORD DEAD LOAD : 3.0 P.S.F.

BOTTOM CHORD LIVE LOAD : 10.5 P.S.F.

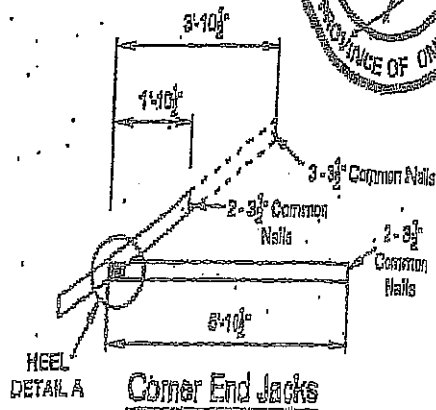
BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD : 59.9 P.S.F.

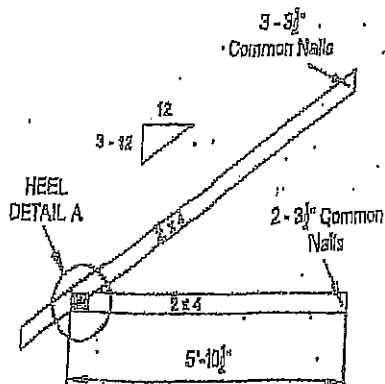
DWG NO. TIM 2495.14
STRUCTURAL
COMPONENT ONLY



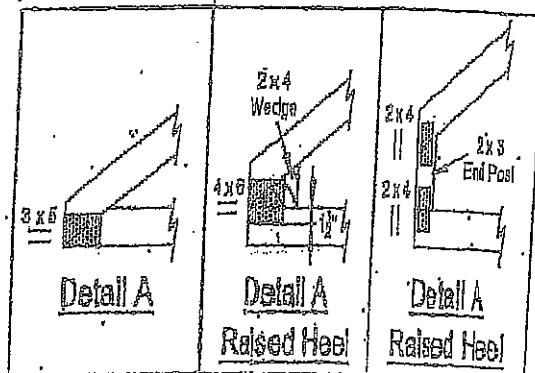
Corner Side Jacks



Corner End Jacks



Common End Jacks



MICRO CITY ENGINEERING SERVICES INC.

TEL: (519) 287-2242

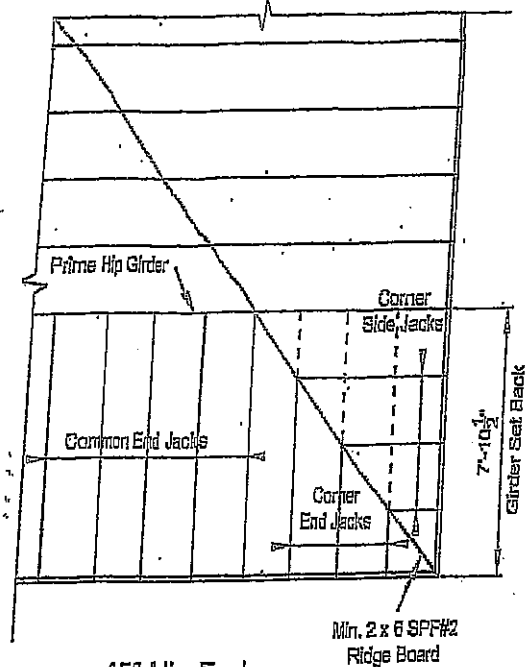
R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

LUMBER SPECIFICATION

TOP CHORD : 2x4 SPF#2
BOTTOM CHORD : 2x4 SPF#2
WEBS : 2x3 SPF#2
UNLESS OTHERWISE SHOWN

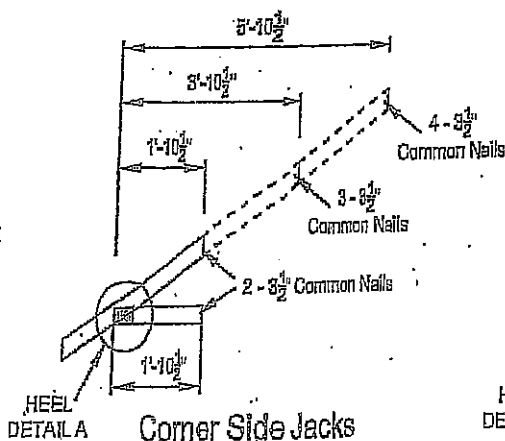
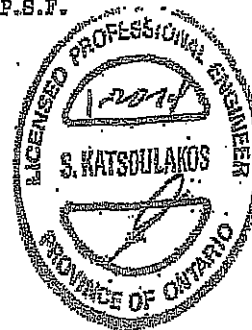
DESIGN LOAD:

TOP CHORD LIVE LOAD : 34.8 P.S.F.
TOP CHORD DEAD LOAD : 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.
TOTAL LOAD : 44.8 P.S.F.

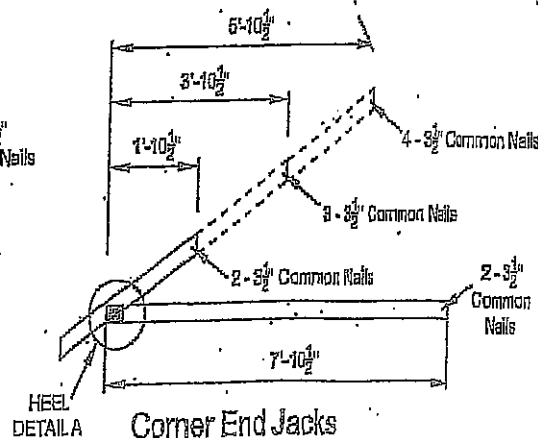


45° Hip End

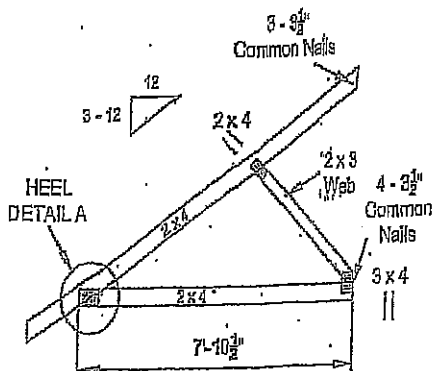
NO. 350314
STRUCTURAL
COMPONENT ONLY



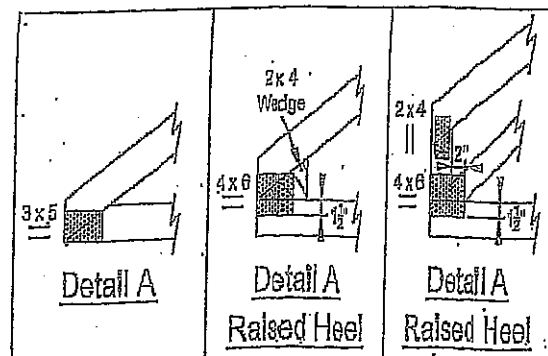
Corner Side Jacks



Corner End Jacks



Common End Jacks

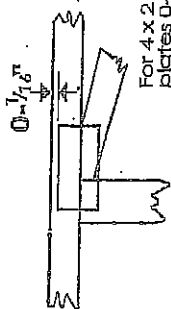


NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

Symbols

PLATE LOCATION AND ORIENTATION

Center plate on joint unless x, y offsets are indicated. Dimensions are in 1/16-in. increments or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/4\"/>

This symbol indicates the required direction of slots in connector plates.

* Plate location details available in Mitek software or upon request.

PLATE SIZE

4 x 4

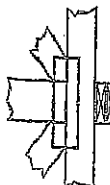
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use 1, 1 or Eliminator bracing if indicated.

BEARING



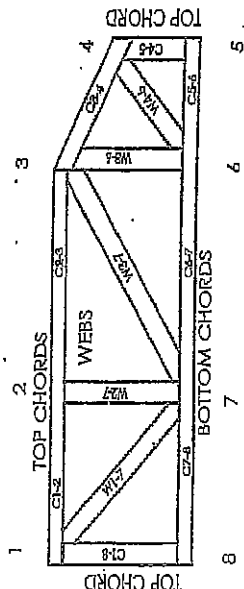
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing
BCSI: Building Component Safety Information, Guide to Good Practices for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in 1/16-in. increments or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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Mitek
POWER TO PERFORM™

Mitek Engineering Reference Sheet: MIP-7473C rev. 10-08

General Safety Notes

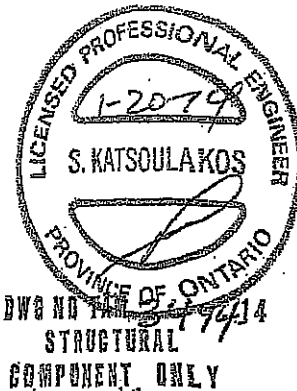
Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral bracing members may require bracing, or alternative 1, 1, or Eliminator bracing should be considered.
3. Never exceed the design loadings shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and ensure full, knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (text, back words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

Micro City Engineering Services Inc.
(BCIN: 26064; FIRM BCIN: 29991)

RR #1, Po Box 61
Glencoe, Ontario
N0L 1M0

(519) 287 - 2242; Fax: (519) 287 - 5750 (Call)



Responsibilities:

Micro City Engineering Services is responsible for the design of trusses as individual components.

It is the responsibilities of others to ascertain that the design loads utilized on this (these) drawing(s) meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdiction over such decisions.

All dimensions are to be verified by the owner, contractor, architect, or other authority having input over such decisions prior to truss component manufacture. At no time shall Micro City Engineering Services Inc. or its employees be responsible for dimension errors.

Micro City Engineering Services Inc. bears no responsibility for the erection of any truss components. Persons erecting truss components are cautioned to seek professional advice regarding temporary and permanent bracing systems and to be totally familiar with all aspects of truss erection prior to proceeding on any truss component erection job. Any bracing shown on Micro City Engineering Services Inc. or Tamarack Roof Trusses Inc. sealed or unsealed truss component drawings is specified for the single truss component in question and is identified as an integral part of the design for that particular truss component but is not meant to represent the only required bracing for that particular truss component when installed as a component in a series of truss components in a roof truss system.

~~It is the truss manufacturer's responsibility to ensure that trusses are manufactured in accordance with Micro City Engineering Services Inc. specifications outlined below:~~

SPECIFICATIONS:

Truss components sealed by Micro City Engineering Services Inc. must conform to the relevant sections of the current Building Code of Ontario and Canada (Part 4 or Part 9) or the current Farm Building Code of Canada in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the Truss Plate Institute of Canada (TPIC). All unit lumber and nailing stresses identified on truss component design drawings and/or used in the design of individual truss components shall conform to the current CSA Wood Design standard identified in the current Building Code and TPIC Design Standards.

The lumber used to manufacture any truss component is to conform to the specified size and grade identified on the truss drawing.

The lumber used in the manufacture of any truss component is not to exceed 19% during its service use unless specifically noted on the truss drawing.

The lumber used in the manufacture of any truss component is not to be treated with any chemicals during its service life unless specifically noted on the truss drawing.

Connector plates shall be applied to both faces of the truss component at each joint and shall be positioned exactly as specified.

The top chord of any truss component is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the sealed truss component drawing but not exceeding 24" o/c (Part 9 design) and not exceeding 48" o/c (Part 4 or Agricultural design).

When a truss component is to be installed with no rigid ceiling attached directly to the bottom chord, then the bottom chord is to be laterally braced at intervals not exceeding 3m (or 10'-0").

All sealed or unsealed truss component drawings provided by Micro City Engineering Services Inc. Or Tamarack Roof Trusses Inc. should be read in conjunction with the following:

Warning-Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473C rev 10-'08 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer - not the truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpinc.ca and BCSI Building Component Safety Information available from the Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA, 22314.