ASPHALT SHINGLES
12"FINISH O.H.
R.T.M.C
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HIGHGROVE-7E
EL:2 - UNIT1
285415

HIGHGROVE-1
EL:2- UNIT2
285416

C OF WOOD PARTY WALL ABOVE CEILING

HIGHGROVE-1
EL:2(REV)- UNIT3
285420

C OF WOOD PARTY WALL ABOVE CEILING

HIGHGROVE-1
EL:2- UNIT4
285424

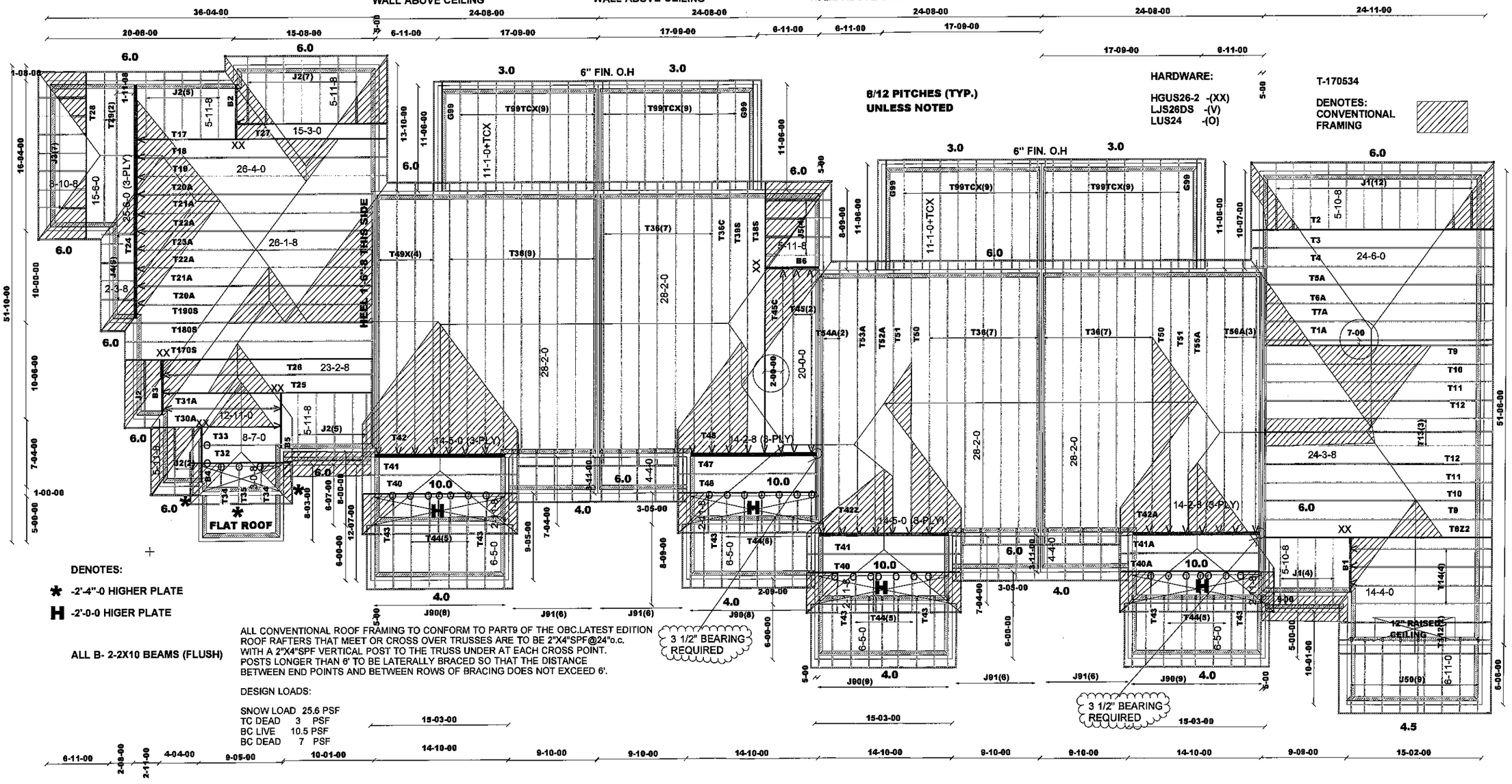
C OF WOOD PARTY WALL ABOVE CEILING

HIGHGROVE-1
EL:2(REV)- UNIT5
285425

C of FIREWALL

HIGHGROVE-5E
EL:2(REV) - UNIT6
285426

C OF WOOD PARTY WALL ABOVE CEILING



ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION
ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"o.c.
WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.
POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE
BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN LOADS:

SNOW LOAD 25.6 PSF
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

M10,988

TAMARACK LUMBER INC. — ALPA LUMBER GROUP

Job Track:	43954
Layout ID:	285414
Plan Log:	93446

Builder / Location: **GREENPARK HOMES / WATERDOWN**
Model / Elevation: **BLOCK204/ UNIT1 TO 6 (REVISED)**
Mitek ver 7.5.0
Project: **RUSSEL GARDENS PHASE 2**
Date: 4/18/2018 Designer: sonny

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

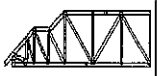
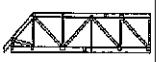
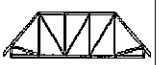
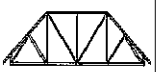
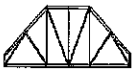
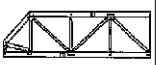
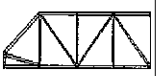
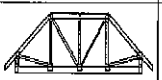
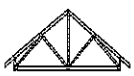
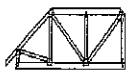
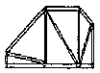
Delivery Shiplist

DATE	08/22/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 285415 LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH 1 SUB-BUILDER:
MODEL: HIGHGROVE 7E ELEVATION: 2 UNIT1

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T170S ROOF	8.00 0.00	26-01-08	05-11-12	2 X 6	2 X 6	01-03-08 00-00-00	01-04-13 05-11-12	173.79 109.33		
	1	T180S ROOF	8.00 0.00	26-01-08	06-11-12	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 06-11-12	118.43 75.00		
	1	T190S ROOF	8.00 0.00	26-01-08	07-11-12	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 07-11-12	128.10 81.33		
	1	T17 HALF HIP	8.00 0.00	26-04-00	04-01-12	2 X 6	2 X 6	01-03-08 00-00-00	01-04-13 04-01-12	151.00 93.67		
	1	T18 HIP	8.00 0.00	26-04-00	05-01-12	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	108.58 68.50		
	1	T19 HIP	8.00 0.00	26-04-00	06-01-12	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	111.24 69.67		
	2	T20A HIP	8.00 0.00	26-01-08	07-01-12	2 X 4	2 X 4	00-00-00 00-00-00	01-04-13 01-06-08	225.90 143.66		
	2	T21A HIP	8.00 0.00	26-01-08	08-01-12	2 X 4	2 X 4	00-00-00 00-00-00	01-04-13 01-06-08	224.72 144.00		
	2	T22A HIP	8.00 0.00	26-01-08	09-01-12	2 X 4	2 X 4	00-00-00 00-00-00	01-04-13 01-06-08	235.28 149.00		
	1	T23A COMMON	8.00 0.00	26-01-08	10-02-02	2 X 4	2 X 4	00-00-00 00-00-00	01-04-13 01-06-08	108.34 69.00		
	1 3 Ply	T24 HIP GIRDER	6.00 0.00	25-06-00	02-11-02	2 X 4	2 X 6	00-00-00 00-00-00	01-02-00 01-02-00	392.85 246.99		
	1	T25 HALF HIP	8.00 0.00	23-02-08	04-01-12	2 X 6	2 X 6	00-00-00 00-00-00	01-04-13 04-01-12	128.68 78.34		
	1	T26 HALF HIP	8.00 0.00	23-02-08	05-01-12	2 X 4	2 X 4	00-00-00 00-00-00	01-04-13 05-01-12	92.43 58.33		
	1	T27 HIP GIRDER	8.00 0.00	15-03-00	04-01-12	2 X 4	2 X 6	01-03-08 01-03-08	01-04-13 01-04-13	74.64 47.00		
	1	T28 HIP GIRDER	6.00 0.00	15-06-00	03-11-13	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	62.74 40.00		
	2	T29 COMMON	6.00 0.00	15-06-00	05-00-08	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	121.68 78.34		
	1	T30A HIP GIRDER	8.00 0.00	12-11-00	04-01-12	2 X 4	2 X 6	01-03-08 00-00-00	01-04-13 03-08-04	73.43 46.33		
	1	T31A HIP	8.00 0.00	12-11-00	05-01-12	2 X 4	2 X 4	00-00-00 00-00-00	01-04-13 03-08-13	56.80 36.17		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	08/22/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 285415 LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH 2 SUB-BUILDER:
MODEL: HIGHGROVE 7E ELEVATION: 2 UNIT1

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	1	T32 HIP GIRDER	8.00 / 0.00	08-07-00	05-09-02	2 X 4	2 X 4	01-03-08 / 01-03-08	03-08-13 / 03-08-13	56.46 / 38.33		
	1	T33 COMMON	8.00 / 0.00	08-07-00	06-07-02	2 X 4	2 X 4	01-03-08 / 01-03-08	03-08-13 / 03-08-13	46.19 / 29.67		
	2	T34 HALF HIP	8.00 / 0.00	03-00-08	02-08-13	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 05-00-13	41.48 / 29.34		
	1	T35 MONOPITCH	8.00 / 0.00	03-00-08	03-05-02	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 00-05-03	18.70 / 12.17		
	20	J2 JACK-OPEN	6.00 / 0.00	05-11-08	04-01-12	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 04-01-12	339.80 / 213.40		
	7	J3 JACK-OPEN	8.00 / 0.00	03-10-08	03-11-13	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 03-11-13	89.67 / 56.00		
	5	J4 JACK-OPEN	8.00 / 0.00	02-03-08	02-11-02	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 02-11-02	44.15 / 30.00		

TOTAL # TRUSS= **61.00** TOTAL BFT OF ALL TRUSSES= **2043.57** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3225.08** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	HGUS26-2	
5	Hangers	LUS24	
16	Hangers	LJS26DS	

TOTAL # ITEMS= **24.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

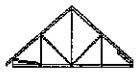
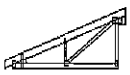
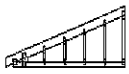
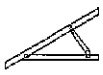
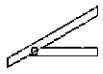
Delivery Shiplist

DATE	08/22/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 285416 LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH 2 SUB-BUILDER:
MODEL: HIGHGROVE 1 ELEVATION: 2 UNIT2

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	9	T36 COMMON	6.00 / 0.00	28-02-00	08-02-08	2 X 4	2 X 4	01-03-08 / 01-03-08	01-02-00 / 01-02-00	991.44 / 618.03		
	4	T49X COMMON	6.00 / 0.00	28-02-00	08-00-08	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 00-10-00	433.16 / 267.32		
	1	T40 HIP GIRDER	8.00 / 0.00	14-05-00	06-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	03-04-13 / 03-04-13	72.45 / 46.00		
	1	T41 HIP	8.00 / 0.00	14-05-00	07-09-04	2 X 4	2 X 4	01-03-08 / 01-03-08	03-04-13 / 03-04-13	83.34 / 52.67		
	1 / 3 Ply	T42 COMMON	8.00 / 0.00	14-05-00	08-02-08	2 X 6	2 X 6	00-00-00 / 00-00-00	03-04-13 / 03-04-13	296.76 / 174.99		
	2	T43 HALF HIP	10.00 / 0.00	02-11-08	02-08-13	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 04-08-13	41.08 / 29.34		
	5	T44 MONOPITCH	10.00 / 0.00	02-11-08	04-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 00-05-13	98.55 / 64.15		
	9	T99TCX MONOPITCH	3.00 / 0.00	11-01-00	03-07-13	2 X 6	2 X 4	00-09-08 / 00-00-00	00-08-04 / 03-07-13	431.37 / 279.00		
	1	G99 GABLE	3.00 / 0.00	11-01-00	03-05-08	2 X 6	2 X 4	00-09-08 / 00-00-00	00-08-04 / 03-05-08	43.96 / 29.33		
	8	J90 JACK	4.00 / 0.00	06-05-00	02-05-10	2 X 4	2 X 4	01-03-08 / 00-00-00	00-03-15 / 02-05-10	145.60 / 93.36		
	6	J91 JACK	4.00 / 0.00	04-04-00	01-09-04	2 X 4	2 X 4	01-03-08 / 00-00-00	00-03-15 / 01-09-04	70.86 / 48.00		

TOTAL # TRUSS= **49.00** TOTAL BFT OF ALL TRUSSES= **1702.19** BFT. TOTAL WEIGHT OF ALL TRUSSES= **2708.57** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
7	Hangers	LJS26DS	
7	Hangers	LUS24	

TOTAL # ITEMS= **14.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

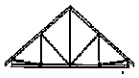
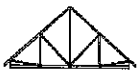
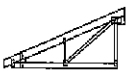
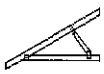
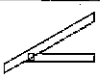
Delivery Shiplist

DATE	08/22/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 285420 LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH2 SUB-BUILDER:
MODEL: HIGHGROVE 1 ELEVATION: 2 (REV)- UNIT3

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	7	T36 COMMON	6.00 0.00	28-02-00	08-02-08	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	771.12 480.69		
	1	T36C COMMON	6.00 0.00	28-02-00	08-00-08	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	109.99 68.67		
	1	T38S ROOF	6.00 0.00	28-02-00	08-11-08	2 X 4	2 X 6	01-03-08 00-00-00	01-02-00 08-11-08	154.15 97.17		
	1	T39S ROOF	6.00 0.00	28-02-00	07-07-08	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 07-07-08	129.54 83.33		
	2	T45 ROOF	6.00 0.00	20-00-00	10-05-07	2 X 4	2 X 4	00-00-00 00-00-00	00-10-08 09-09-00	198.18 125.34		
	1	T45C ROOF	6.00 0.00	20-00-00	10-03-08	2 X 4	2 X 4	00-00-00 00-00-00	00-10-08 09-09-00	98.89 62.67		
	1	T46 HALF HIP	8.00 0.00	14-02-08	06-01-04	2 X 4	2 X 4	01-03-08 00-00-00	03-04-13 06-01-04	69.72 44.50		
	1	T47 HALF HIP	8.00 0.00	14-02-08	07-09-04	2 X 4	2 X 4	01-03-08 00-00-00	03-04-13 07-09-04	75.97 48.33		
	1 3 Ply	T48 HALF HIP	8.00 0.00	14-02-08	09-05-04	2 X 6	2 X 6	00-00-00 00-00-00	03-04-13 09-05-04	311.43 194.01		
	1	T43 HALF HIP	10.00 0.00	02-11-08	02-08-13	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 04-08-13	20.54 14.67		
	6	T44 MONOPITCH	10.00 0.00	02-11-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 00-05-13	118.26 76.98		
	9	T99TCX MONOPITCH	3.00 0.00	11-01-00	03-07-13	2 X 6	2 X 4	00-09-08 00-00-00	00-08-04 03-07-13	431.37 279.00		
	1	G99 GABLE	3.00 0.00	11-01-00	03-05-08	2 X 6	2 X 4	00-09-08 00-00-00	00-08-04 03-05-08	43.96 29.33		
	4	J5 JACK-OPEN	8.00 0.00	05-11-08	05-04-08	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-04-08	72.16 42.68		
	8	J90 JACK	4.00 0.00	06-05-00	02-05-10	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-05-10	145.60 93.36		
	6	J91 JACK	4.00 0.00	04-04-00	01-09-04	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 01-09-04	70.86 48.00		

TOTAL # TRUSS= **53.00** TOTAL BFT OF ALL TRUSSES= **1788.73** BFT. TOTAL WEIGHT OF ALL TRUSSES= **2821.74** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	08/22/17
SALES REP	Mario

JOB TRACK: 43954 **LAYOUT ID:** 285420 **LOCATION:**
BUILDER: GREENPARK - RUSSELL GARDENS PH1 **SUB-BUILDER:**
MODEL: HIGHGROVE 1 **ELEVATION:** 2 (REV)- UNIT3

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	HGUS26-2	
11	Hangers	LJS26DS	
7	Hangers	LUS24	

TOTAL # ITEMS= **19.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

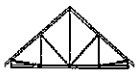
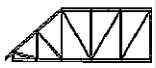
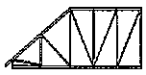
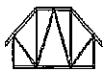
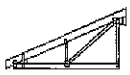
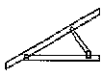
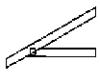
Delivery Shiplist

DATE	08/22/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 285424 LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH 2 SUB-BUILDER:
MODEL: HIGHGROVE 1 ELEVATION: 2 UNIT4

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	7	T36 COMMON	6.00 0.00	28-02-00	08-02-08	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	771.12 480.69		
	1	T50 HALF HIP	6.00 0.00	28-02-00	06-01-13	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 06-01-13	118.79 75.33		
	1	T51 HALF HIP	6.00 0.00	28-02-00	07-05-13	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 07-05-13	127.83 79.50		
	1	T52A ROOF	6.00 0.00	28-02-00	08-09-13	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 02-05-08	122.14 78.00		
	1	T53A ROOF	6.00 0.00	28-02-00	10-01-13	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 02-05-08	132.58 83.50		
	2	T54A ROOF	6.00 0.00	28-02-00	10-05-07	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 02-05-08	242.58 151.00		
	1	T40 HIP GIRDER	8.00 0.00	14-05-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	03-04-13 03-04-13	72.45 46.00		
	1	T41 HIP	8.00 0.00	14-05-00	07-09-04	2 X 4	2 X 4	01-03-08 01-03-08	03-04-13 03-04-13	83.34 52.67		
	1 3 Ply	T42Z COMMON	8.00 0.00	14-05-00	08-02-08	2 X 6	2 X 6	00-00-00 00-00-00	03-04-13 03-04-13	296.76 174.99		
	2	T43 HALF HIP	10.00 0.00	02-11-08	02-08-13	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 04-08-13	41.08 29.34		
	5	T44 MONOPITCH	10.00 0.00	02-11-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 00-05-13	98.55 64.15		
	9	T99TCX MONOPITCH	3.00 0.00	11-01-00	03-07-13	2 X 6	2 X 4	00-09-08 00-00-00	00-08-04 03-07-13	431.37 279.00		
	1	G99 GABLE	3.00 0.00	11-01-00	03-05-08	2 X 6	2 X 4	00-09-08 00-00-00	00-08-04 03-05-08	43.96 29.33		
	9	J90 JACK	4.00 0.00	06-05-00	02-05-10	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-05-10	163.80 105.03		
	6	J91 JACK	4.00 0.00	04-04-00	01-09-04	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 01-09-04	70.86 48.00		

TOTAL # TRUSS= **50.00** TOTAL BFT OF ALL TRUSSES= **1776.53** BFT. TOTAL WEIGHT OF ALL TRUSSES= **2817.21** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
7	Hangers	LJS26DS	
7	Hangers	LUS24	

TOTAL # ITEMS= **14.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

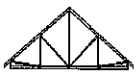
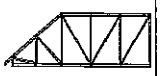
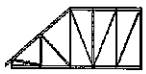
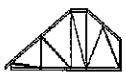
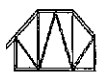
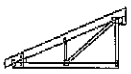
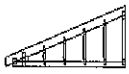
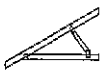
Delivery Shiplist

DATE	08/22/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 285425 LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH2 SUB-BUILDER:
MODEL: HIGHGROVE 1 ELEVATION: 2(REV)- UNIT5

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	7	T36 COMMON	6.00 / 0.00	28-02-00	08-02-08	2 X 4	2 X 4	01-03-08 / 01-03-08	01-02-00 / 01-02-00	771.12 / 480.69		
	1	T50 HALF HIP	6.00 / 0.00	28-02-00	06-01-13	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 06-01-13	118.79 / 75.33		
	1	T51 HALF HIP	6.00 / 0.00	28-02-00	07-05-13	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 07-05-13	127.83 / 79.50		
	1	T55A HIP	6.00 / 0.00	28-02-00	08-09-13	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 04-05-08	124.35 / 78.33		
	3	T56A COMMON	6.00 / 0.00	28-02-00	09-10-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 04-05-08	362.28 / 226.50		
	1	T40A HIP GIRDER	8.00 / 0.00	14-02-08	06-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	03-04-13 / 03-06-08	70.26 / 45.33		
	1	T41A HIP	8.00 / 0.00	14-02-08	07-09-04	2 X 4	2 X 4	01-03-08 / 00-00-00	03-04-13 / 03-06-08	81.11 / 52.00		
	1 / 3 Ply	T42A COMMON	8.00 / 0.00	14-02-08	08-02-08	2 X 6	2 X 6	00-00-00 / 00-00-00	03-04-13 / 03-06-08	294.36 / 174.99		
	2	T43 HALF HIP	10.00 / 0.00	02-11-08	02-08-13	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 04-08-13	41.08 / 29.34		
	5	T44 MONOPITCH	10.00 / 0.00	02-11-08	04-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 00-05-13	98.55 / 64.15		
	9	T99TCX MONOPITCH	3.00 / 0.00	11-01-00	03-07-13	2 X 6	2 X 4	00-09-08 / 00-00-00	00-08-04 / 03-07-13	431.37 / 279.00		
	1	G99 GABLE	3.00 / 0.00	11-01-00	03-05-08	2 X 6	2 X 4	00-09-08 / 00-00-00	00-08-04 / 03-05-08	43.96 / 29.33		
	9	J90 JACK	4.00 / 0.00	06-05-00	02-05-10	2 X 4	2 X 4	01-03-08 / 00-00-00	00-03-15 / 02-05-10	163.80 / 105.03		
	6	J91 JACK	4.00 / 0.00	04-04-00	01-09-04	2 X 4	2 X 4	01-03-08 / 00-00-00	00-03-15 / 01-09-04	70.86 / 48.00		

TOTAL # TRUSS= **50.00** TOTAL BFT OF ALL TRUSSES= **1767.52** BFT. TOTAL WEIGHT OF ALL TRUSSES= **2799.72** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
7	Hangers	LJS26DS	
7	Hangers	LUS24	

TOTAL # ITEMS= **14.00**

TAMARACK LUMBER INC
ALPA LUMBER GROUP

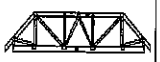
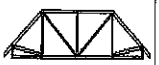
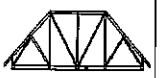
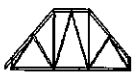
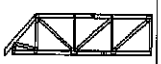
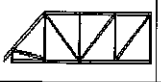
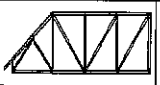
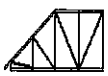
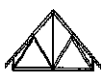
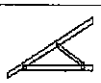
Delivery Shiplist

DATE	04/18/18
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 285426 LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH 2 SUB-BUILDER:
MODEL: HIGHGROVE 5E ELEVATION: 2(REV)-UNIT6

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T1A COMMON	8.00 0.00	24-03-08	09-06-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	102.41 64.00		
	1	T2 HIP GIRDER	8.00 0.00	24-06-00	04-01-04	2 X 6	2 X 6	01-03-08 01-01-00	01-04-13 01-04-13	145.29 89.67		
	1	T3 HIP	8.00 0.00	24-06-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	97.09 60.83		
	1	T4 HIP	8.00 0.00	24-06-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	105.79 67.00		
	1	T5A HIP	8.00 0.00	24-03-08	07-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	104.32 67.00		
	1	T6A HIP	8.00 0.00	24-03-08	08-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	108.82 69.50		
	1	T7A HIP	8.00 0.00	24-03-08	09-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	114.67 72.50		
	1	T8Z2 HALF HIP	8.00 0.00	24-03-08	04-01-04	2 X 6	2 X 6	01-03-08 00-00-00	01-04-13 04-01-04	138.05 86.67		
	2	T9 HALF HIP	8.00 0.00	24-03-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-01-04	197.10 123.66		
	2	T10 HALF HIP	8.00 0.00	24-03-08	06-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 06-01-04	212.66 133.34		
	2	T11 HALF HIP	8.00 0.00	24-03-08	07-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 07-01-04	225.56 142.68		
	2	T12 HALF HIP	8.00 0.00	24-03-08	08-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 08-01-04	234.64 146.34		
	3	T13 HALF HIP	8.00 0.00	24-03-08	09-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 09-01-04	368.64 227.49		
	4	T14 COMMON	8.00 0.00	14-04-00	06-02-02	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	247.04 156.00		
	2	T112 ROOF	8.00 0.00	14-04-00	06-02-02	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	135.80 89.00		
	16	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	268.64 170.72		
	9	J50 JACK	4.50 0.00	06-11-00	02-11-12	2 X 4	2 X 4	01-03-08 00-00-00	00-04-10 00-05-13	183.24 109.53		

TOTAL # TRUSS= **50.00** TOTAL BFT OF ALL TRUSSES= **1875.93** BFT. TOTAL WEIGHT OF ALL TRUSSES= **2989.76** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16

TAMARACK LUMBER INC
ALPA LUMBER GROUP

Delivery Shiplist

DATE	04/18/18
SALES REP	Mario

JOB TRACK: 43954 **LAYOUT ID:** 285426 **LOCATION:**
BUILDER: GREENPARK - RUSSELL GARDENS PH2 **SUB-BUILDER:**
MODEL: HIGHGROVE 5E **ELEVATION:** 2(REV)-UNIT6

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	HGUS26-2	
3	Hangers	LJS26DS	

TOTAL # ITEMS= **4.00**

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43994	DRWG NO.
285994	T1A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Dec 12 2017 MiTek Industries, Inc. Tue Jan 16 10:45:48 2018 Page 1

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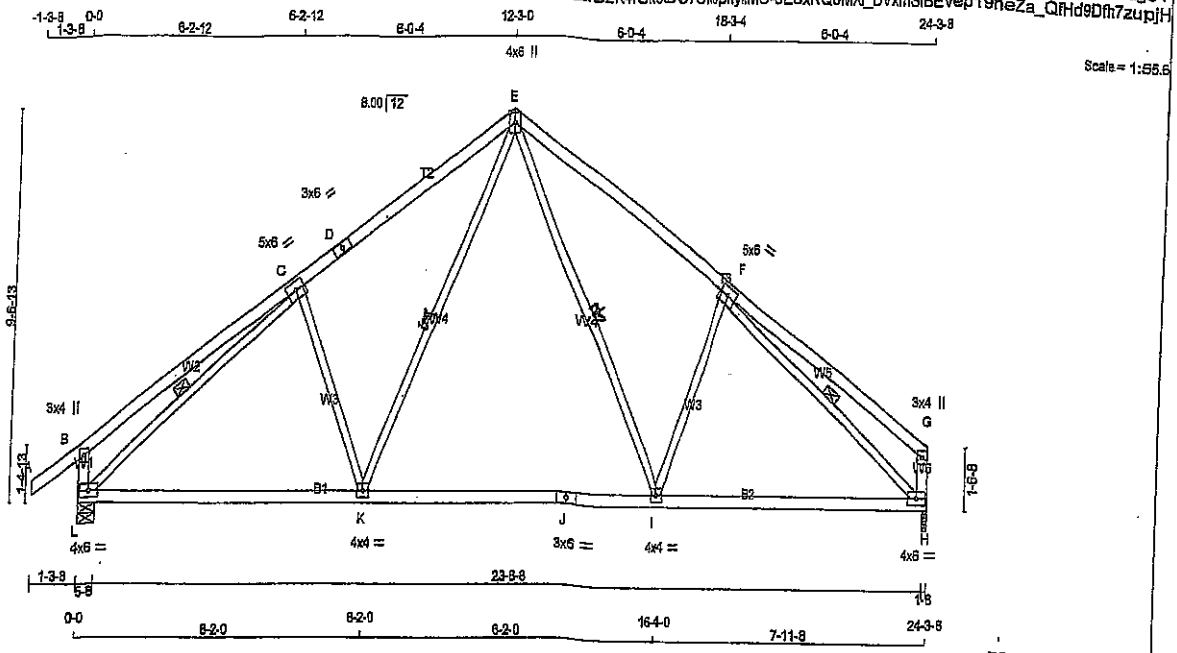

TOTAL WEIGHT = 102 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESGR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TS-t	MT20	3.0	6.0		
E	TTWW+p	MT20	4.0	6.0	Edge	
F	TMWW-t	MT20	5.0	6.0		
G	TMV+p	MT20	3.0	4.0		
H	BMWW1-t	MT20	4.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1479	0	1479	0	0	5-8	5-8
H	1363	0	1363	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1198	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0
H	1119	521 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.96 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-I, F-H. EN, BI

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00
B-C	0 / 34	-84.3	-84.3	0.52 (1)	10.00
C-D	-1409 / 0	-84.3	-84.3	0.45 (1)	4.96
D-E	-1409 / 0	-84.3	-84.3	0.45 (1)	4.96
E-F	-1388 / 0	-84.3	-84.3	0.44 (1)	5.01
F-G	0 / 34	-84.3	-84.3	0.50 (1)	10.00
L-B	-311 / 0	0.0	0.0	0.03 (1)	7.81
H-G	-186 / 0	0.0	0.0	0.02 (1)	7.81
L-K	0 / 1275	-28.0	-28.0	0.58 (2)	10.00
K-J	0 / 869	-28.0	-28.0	0.54 (2)	10.00
J-I	0 / 869	-28.0	-28.0	0.54 (2)	10.00
I-H	0 / 1245	-28.0	-28.0	0.56 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-K	-352 / 44	0.19 (1)
K-E	0 / 632	0.14 (1)
E-I	0 / 569	0.13 (1)
I-F	-316 / 58	0.17 (1)
L-C	-1669 / 0	0.71 (1)
F-H	-1653 / 0	0.67 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(65 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.29")

CSI: TC=0.52/1.00 (B-C:1) , BC=0.58/1.00 (K-L:2) , WB=0.71/1.00 (C-L:1) , SSI=0.21/1.00 (C-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1657	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (L) (INPUT = 0.90)
JSI METAL= 0.43 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
1-767B
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM3984-18
STRUCTURAL

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43854	DRWG NO.
285414	T2	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Wed Aug 23 09:35:20 2017 Page 1

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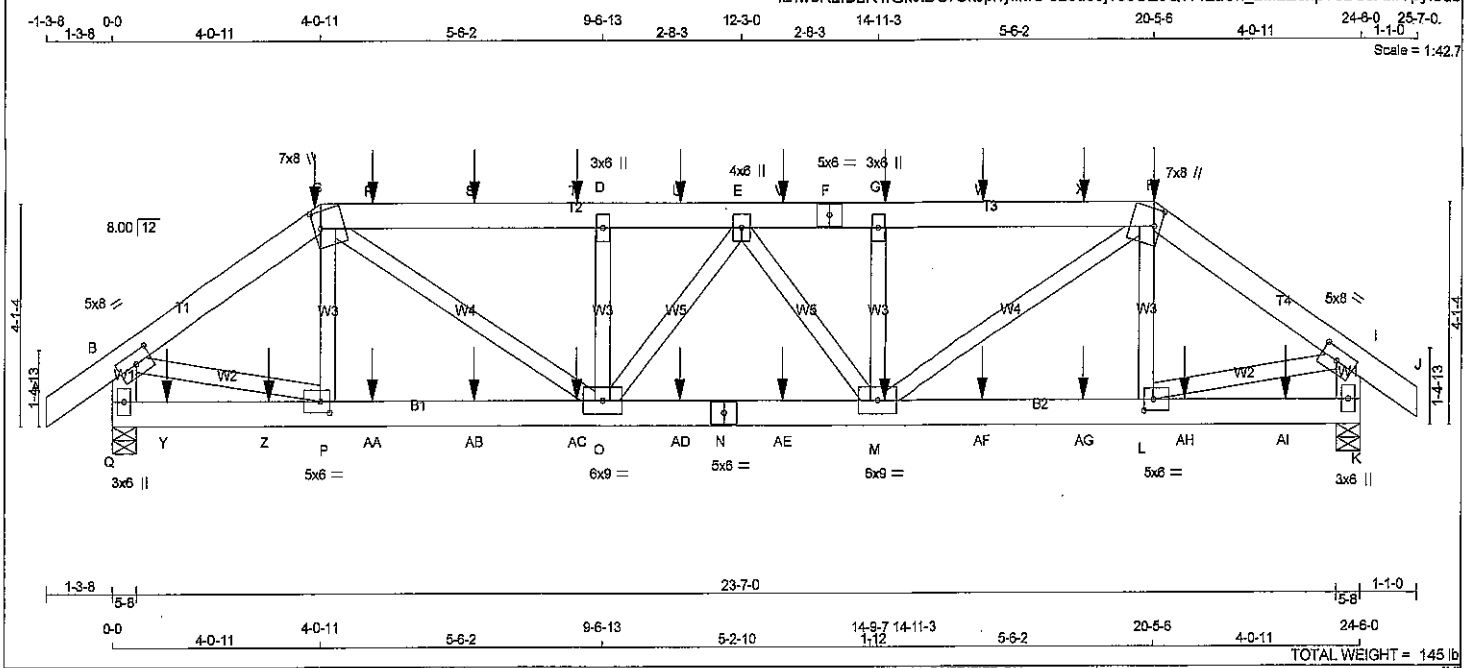


TOTAL WEIGHT = 145 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x8	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x8	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
Q - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
Q - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.50	3.75
C	TTWW+m	MT20	7.0	8.0	3.75	1.50
D	TMW+w	MT20	3.0	6.0		
E	TMWW+t	MT20	4.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMW+w	MT20	3.0	6.0		
H	TTWW+m	MT20	7.0	8.0	3.75	1.50
I	TMVW-t	MT20	5.0	8.0	2.50	3.75
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.25
M	BMWWW-t	MT20	6.0	9.0		
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	6.0	9.0		
P	BMWW-t	MT20	5.0	6.0	2.50	2.25
Q	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 244.5 lbs FACTORED DOWN AT 4-0-11, 244.5 lbs FACTORED DOWN AT 20-5-5, 101.4 lbs FACTORED DOWN AT 5-0-12, 101.4 lbs FACTORED DOWN AT 7-0-12, 101.4 lbs FACTORED DOWN AT 9-0-12, 101.4 lbs FACTORED DOWN AT 11-0-12, 101.4 lbs FACTORED DOWN AT 13-0-12, 101.4 lbs FACTORED DOWN AT 15-0-12, AND 101.4 lbs FACTORED DOWN AT 17-0-12, AND 101.4 lbs FACTORED DOWN AT 19-0-12 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 1-0-12, 69.9 lbs FACTORED DOWN AT 3-0-12, 69.9 lbs FACTORED DOWN AT 5-0-12, 69.9 lbs FACTORED DOWN AT 7-0-12, 69.9 lbs FACTORED DOWN AT 9-0-12, 69.9 lbs FACTORED DOWN AT 11-0-12, 69.9 lbs FACTORED DOWN AT 13-0-12, 69.9 lbs FACTORED DOWN AT 15-0-12, 69.9 lbs FACTORED DOWN AT 17-0-12, 69.9 lbs FACTORED DOWN AT 19-0-12, AND 69.9 lbs FACTORED DOWN AT 21-0-12, AND 69.9 lbs FACTORED DOWN AT 23-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
Q	2395	0	2395	0	0	5-8	5-8
K	2356	0	2356	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1960	1098 / 0	440 / 0	0 / 0	0 / 0	423 / 0	0 / 0
K	1930	1079 / 0	434 / 0	0 / 0	0 / 0	417 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

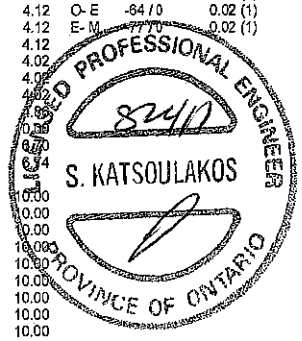
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF) CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FORCE (LBS)	MAX CSI (LC)
A-B	0 / 33	-84.3	-84.3	0.07 (1)	10.00	P-C	-226 / 200	0.04 (1)
B-C	-2761 / 0	-84.3	-84.3	0.17 (1)	4.85	C-O	0 / 1952	0.33 (1)
C-R	-3829 / 0	-84.3	-84.3	0.39 (1)	4.00	O-D	-717 / 0	0.12 (1)
R-S	-3829 / 0	-84.3	-84.3	0.39 (1)	4.00	M-G	-713 / 0	0.12 (1)
S-T	-3829 / 0	-84.3	-84.3	0.39 (1)	4.00	M-H	0 / 1862	0.33 (1)
T-D	-3829 / 0	-84.3	-84.3	0.39 (1)	4.00	L-H	-223 / 199	0.04 (1)
D-U	-3830 / 0	-84.3	-84.3	0.30 (1)	4.10	B-P	0 / 2343	0.41 (1)
U-E	-3830 / 0	-84.3	-84.3	0.30 (1)	4.10	L-I	0 / 2326	0.41 (1)
E-V	-3822 / 0	-84.3	-84.3	0.28 (1)	4.12	O-E	-64 / 0	0.02 (1)
V-F	-3822 / 0	-84.3	-84.3	0.28 (1)	4.12	E-M	-77 / 0	0.02 (1)
F-G	-3822 / 0	-84.3	-84.3	0.28 (1)	4.12			
G-W	-3822 / 0	-84.3	-84.3	0.38 (1)	4.02			
W-X	-3822 / 0	-84.3	-84.3	0.38 (1)	4.02			
X-H	-3822 / 0	-84.3	-84.3	0.38 (1)	4.02			
H-I	-2742 / 0	-84.3	-84.3	0.17 (1)	4.85			
I-J	0 / 26	-84.3	-84.3	0.05 (1)	0.00			
Q-B	-2320 / 0	0.0	0.0	0.16 (1)	6.74			
K-I	-2288 / 0	0.0	0.0	0.16 (1)	6.74			
Q-Y	0 / 0	-28.0	-28.0	0.14 (3)	10.00			
Y-Z	0 / 0	-28.0	-28.0	0.14 (3)	10.00			
Z-P	0 / 0	-28.0	-28.0	0.14 (3)	10.00			
P-AA	0 / 2282	-28.0	-28.0	0.37 (1)	10.00			
AA-AB	0 / 2282	-28.0	-28.0	0.37 (1)	10.00			
AB-AC	0 / 2282	-28.0	-28.0	0.37 (1)	10.00			
AC-O	0 / 2282	-28.0	-28.0	0.37 (1)	10.00			
O-AD	0 / 3868	-28.0	-28.0	0.59 (1)	10.00			
AD-N	0 / 3868	-28.0	-28.0	0.59 (1)	10.00			
N-AE	0 / 3868	-28.0	-28.0	0.59 (1)	10.00			
AE-M	0 / 3868	-28.0	-28.0	0.59 (1)	10.00			
M-AF	0 / 2266	-28.0	-28.0	0.37 (1)	10.00			
AF-AG	0 / 2266	-28.0	-28.0	0.37 (1)	10.00			
AG-L	0 / 2266	-28.0	-28.0	0.37 (1)	10.00			
L-AH	0 / 0	-28.0	-28.0	0.14 (3)	10.00			
AH-AI	0 / 0	-28.0	-28.0	0.14 (3)	10.00			
AI-K	0 / 0	-28.0	-28.0	0.14 (3)	10.00			



DWG NO. TAM 4/8/17
STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.82")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.82")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.39 (C-D:1) , BC=0.59 (M-O:1) , WB=0.41 (B-P:1) , SSI=0.30 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (C) (INPUT = 0.90)
JSI METAL= 0.67 (N) (INPUT = 1.00)

PGL

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285414	T2	1	1	TRUSS DESC.	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	4-0-11	-23	-25	—	FRONT	VERT	DEAD
C	4-0-11	-222	-222	—	FRONT	VERT	SNOW
G	15-0-12	-101	-101	—	FRONT	VERT	TOTAL
H	20-5-5	-23	-25	—	FRONT	VERT	DEAD
H	20-5-5	-222	-222	—	FRONT	VERT	SNOW
M	15-0-12	-40	-70	—	FRONT	VERT	TOTAL
R	5-0-12	-101	-101	—	FRONT	VERT	TOTAL
S	7-0-12	-101	-101	—	FRONT	VERT	TOTAL
T	9-0-12	-101	-101	—	FRONT	VERT	TOTAL
U	11-0-12	-101	-101	—	FRONT	VERT	TOTAL
V	13-0-12	-101	-101	—	FRONT	VERT	TOTAL
W	17-0-12	-101	-101	—	FRONT	VERT	TOTAL
X	19-0-12	-101	-101	—	FRONT	VERT	TOTAL
Y	1-0-12	-40	-70	—	FRONT	VERT	TOTAL
Z	3-0-12	-40	-70	—	FRONT	VERT	TOTAL
AA	5-0-12	-40	-70	—	FRONT	VERT	TOTAL
AB	7-0-12	-40	-70	—	FRONT	VERT	TOTAL
AC	9-0-12	-40	-70	—	FRONT	VERT	TOTAL
AD	11-0-12	-40	-70	—	FRONT	VERT	TOTAL
AE	13-0-12	-40	-70	—	FRONT	VERT	TOTAL
AF	17-0-12	-40	-70	—	FRONT	VERT	TOTAL
AG	19-0-12	-40	-70	—	FRONT	VERT	TOTAL
AH	21-0-12	-40	-70	—	FRONT	VERT	TOTAL
AI	23-0-12	-40	-70	—	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM4/814-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285414	T3	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Wed Aug 23 09:35:20 2017 Page 1

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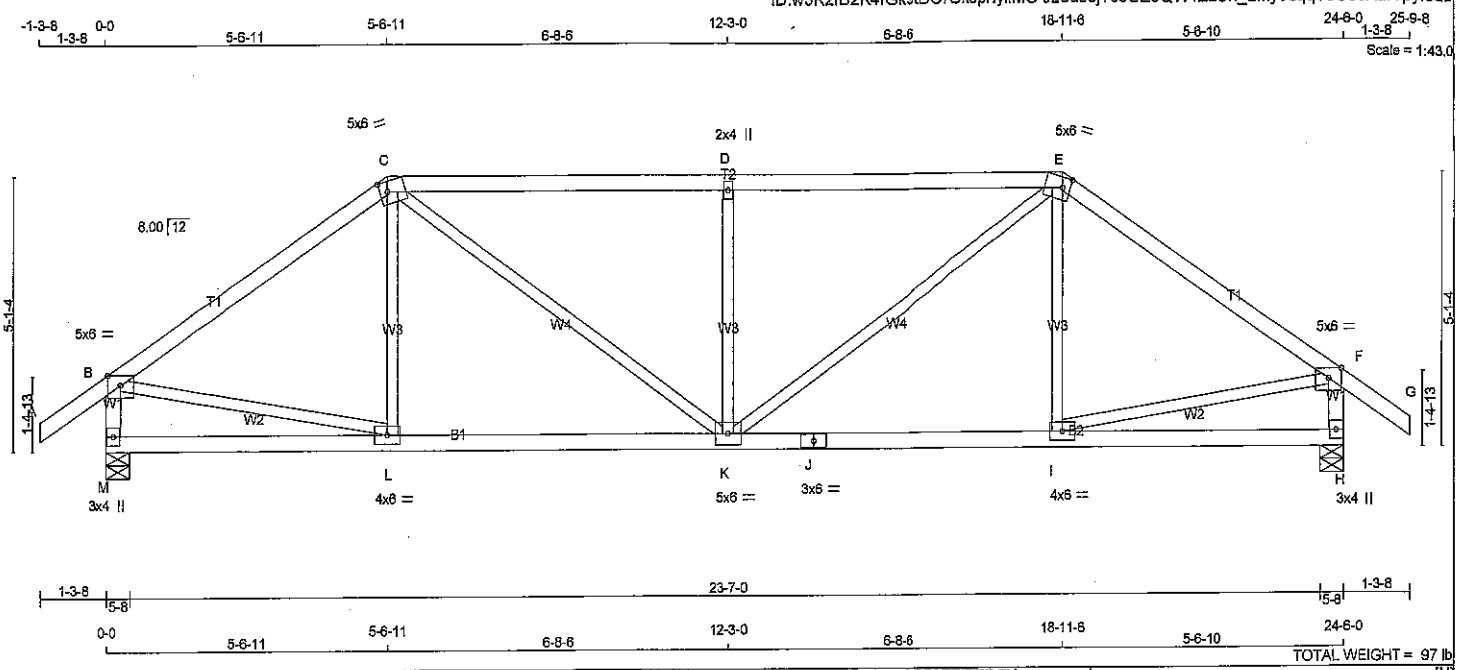


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TTWW-m	MT20	5.0	6.0	2.25	1.75
D	TMW+w	MT20	2.0	4.0		
E	TTWW-m	MT20	5.0	6.0	2.25	1.75
F	TMVW-p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	5.0	6.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1491	0	1491	0	0	5-8	5-8
H	1491	0	1491	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1208	697 / 0	257 / 0	0 / 0	0 / 0	253 / 0	0 / 0
H	1208	697 / 0	257 / 0	0 / 0	0 / 0	253 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.85 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

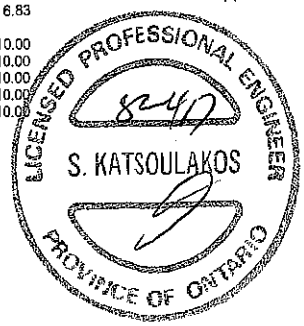
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00
B- C	-1511 / 0	-84.3	-84.3	0.55 (1)	4.67
C- D	-1862 / 0	-84.3	-84.3	0.80 (1)	3.85
D- E	-1862 / 0	-84.3	-84.3	0.80 (1)	3.85
E- F	-1511 / 0	-84.3	-84.3	0.55 (1)	4.67
F- G	0 / 32	-84.3	-84.3	0.11 (1)	10.00
M- B	-1430 / 0	0.0	0.0	0.15 (1)	6.83
H- F	-1430 / 0	0.0	0.0	0.15 (1)	6.83
M- L	0 / 0	-28.0	-28.0	0.25 (3)	10.00
L- K	0 / 1255	-28.0	-28.0	0.40 (2)	10.00
K- J	0 / 1255	-28.0	-28.0	0.40 (2)	10.00
J- I	0 / 1255	-28.0	-28.0	0.40 (2)	10.00
I- H	0 / 0	-28.0	-28.0	0.25 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- C	-47 / 210	0.05 (3)
K- D	-692 / 0	0.27 (1)
K- E	0 / 747	0.17 (1)
I- E	-47 / 210	0.05 (3)
B- L	0 / 1278	0.29 (1)
I- F	0 / 1278	0.29 (1)



DWG NO. T3M 4/0815-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.82")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.82")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.80 (C-D:1) , BC=0.40 (K-L:2) , WB=0.29 (B-L:1) , SSI=0.27 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (L) (INPUT = 0.90)
JSI METAL= 0.37 (J) (INPUT = 1.00)

[M]

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285414	T4	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Wed Aug 23 09:35:21 2017 Page 1
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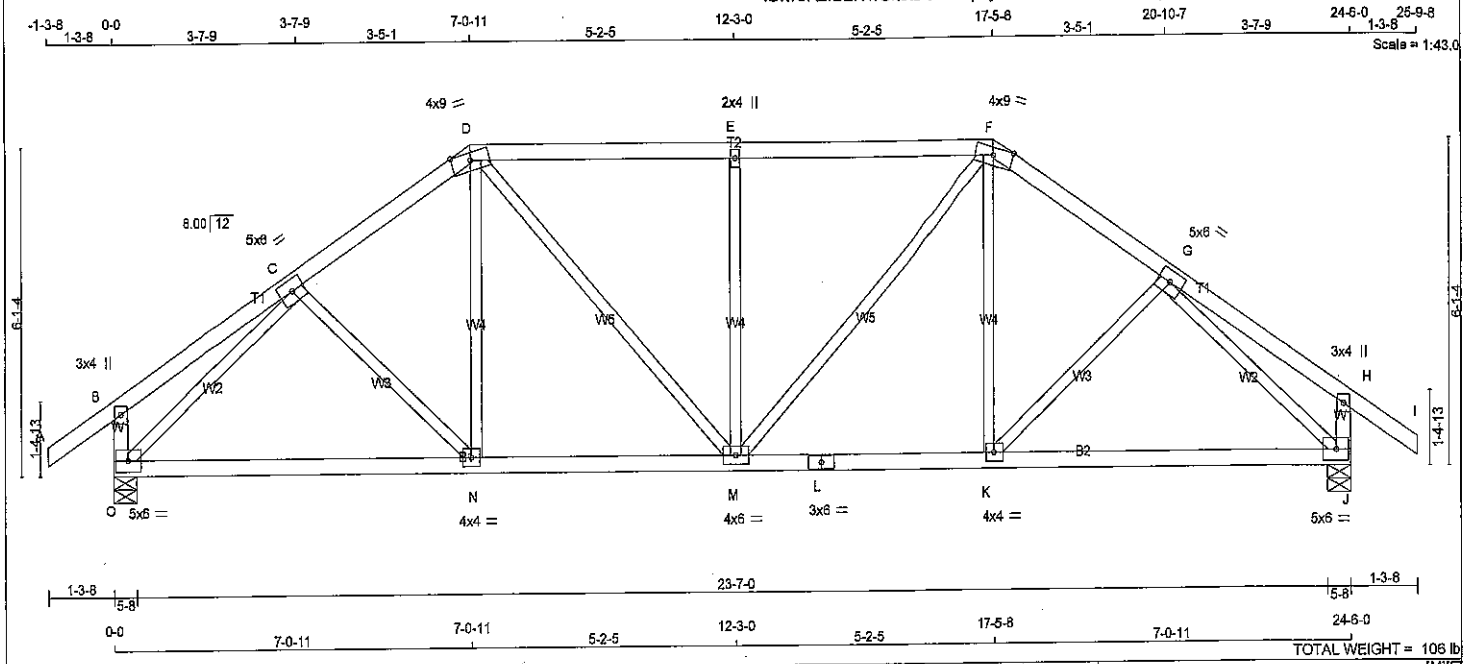


TOTAL WEIGHT = 106 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW-m	MT20	4.0	9.0	Edge	
E	TMV+w	MT20	2.0	4.0		
F	TTWW-m	MT20	4.0	9.0	Edge	
G	TMWW-t	MT20	5.0	6.0		
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	5.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMVW1-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1491	0	1491	0	0	5-8	5-8
J	1491	0	1491	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1208	697 / 0	257 / 0	0 / 0	0 / 0	253 / 0	0 / 0
J	1208	697 / 0	257 / 0	0 / 0	0 / 0	253 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.02 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

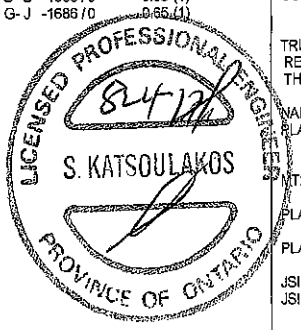
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	
B- C	0 / 17	-84.3	-84.3	0.15 (1)	10.00	
C- D	-1476 / 0	-84.3	-84.3	0.14 (1)	5.27	
D- E	-1498 / 0	-84.3	-84.3	0.32 (1)	5.02	
E- F	-1498 / 0	-84.3	-84.3	0.32 (1)	5.02	
F- G	-1476 / 0	-84.3	-84.3	0.14 (1)	5.27	
G- H	0 / 17	-84.3	-84.3	0.15 (1)	10.00	
H- I	0 / 32	-84.3	-84.3	0.11 (1)	10.00	
O- B	-233 / 0	0.0	0.0	0.02 (1)	7.81	
J- H	-233 / 0	0.0	0.0	0.02 (1)	7.81	
O- N	0 / 1208	-28.0	-28.0	0.45 (2)	10.00	
N- M	0 / 1216	-28.0	-28.0	0.47 (2)	10.00	
M- L	0 / 1216	-28.0	-28.0	0.47 (2)	10.00	
L- K	0 / 1216	-28.0	-28.0	0.47 (2)	10.00	
K- J	0 / 1208	-28.0	-28.0	0.45 (2)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- N	0 / 119	0.03 (3)
N- D	0 / 304	0.07 (2)
D- M	0 / 424	0.10 (1)
M- E	-533 / 0	0.31 (1)
M- F	0 / 424	0.10 (1)
K- F	0 / 304	0.07 (2)
K- G	0 / 119	0.03 (3)
O- C	-1686 / 0	0.65 (1)
G- J	-1686 / 0	0.65 (1)



DWG NO. TAM 4/8/16-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.82")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.82")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.32 (D-E:1) , BC=0.47 (M-N:2) , WB=0.65 (C-O:1) , SSI=0.21 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (F) (INPUT = 0.90)
JSI METAL= 0.41 (C) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285994	T5A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Dec 12 2017 MiTek Industries, Inc. Tue Jan 16 10:45:48 2018 Page 1

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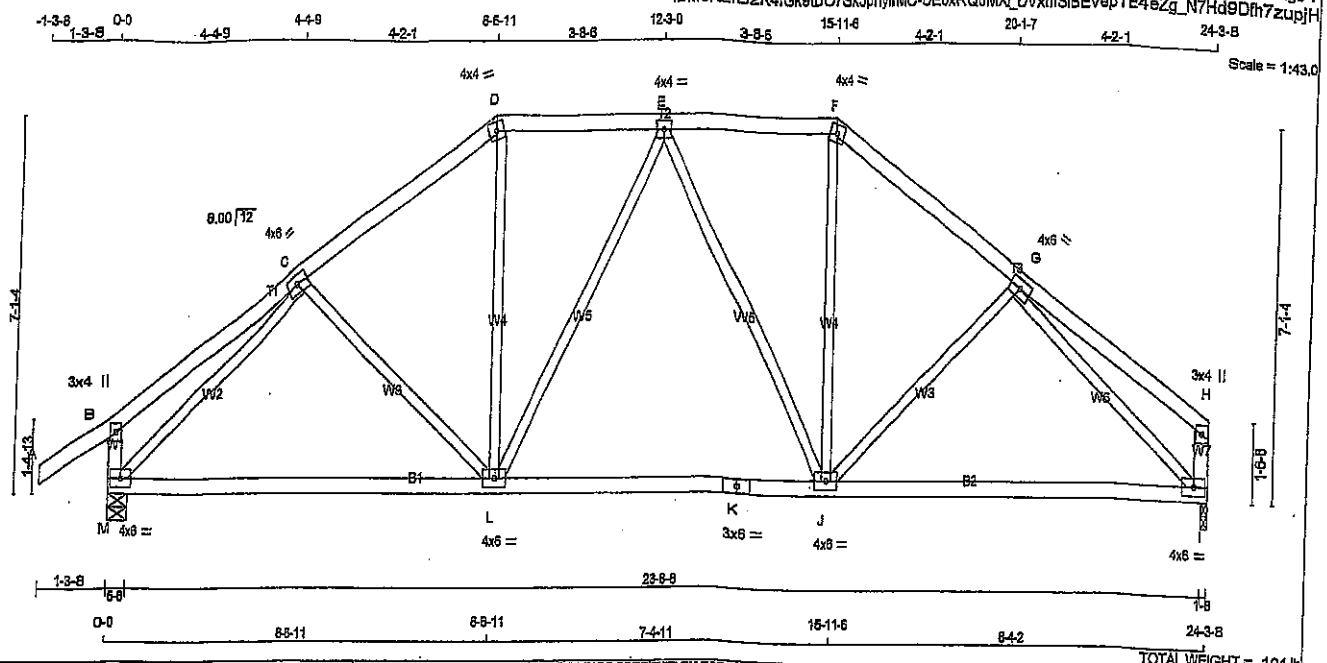

TOTAL WEIGHT = 104 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESGR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	4.0		
G	TMWW-t	MT20	4.0	6.0		
H	TMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	6.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	4.0	6.0		
M	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1479	0	1479	0	0	5-8	5-8
I	1363	0	1363	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1198	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0
I	1119	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.32 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00
B-C	0 / 22	-84.3	-84.3	0.24 (1)	10.00
C-D	-1388 / 0	-84.3	-84.3	0.21 (1)	5.32
D-E	-1142 / 0	-84.3	-84.3	0.15 (1)	5.79
E-F	-1128 / 0	-84.3	-84.3	0.15 (1)	5.82
F-G	-1371 / 0	-84.3	-84.3	0.20 (1)	5.35
G-H	0 / 22	-84.3	-84.3	0.23 (1)	10.00
M-B	-255 / 0	0.0	0.0	0.03 (1)	7.81
I-H	-131 / 0	0.0	0.0	0.01 (1)	7.81
M-L	0 / 1222	-28.0	-28.0	0.57 (2)	10.00
L-K	0 / 1233	-28.0	-28.0	0.58 (2)	10.00
K-J	0 / 1233	-28.0	-28.0	0.59 (2)	10.00
J-I	0 / 1173	-28.0	-28.0	0.54 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-L	-117 / 85	0.07 (1)
L-D	0 / 495	0.11 (1)
L-E	-195 / 0	0.24 (1)
E-J	-225 / 0	0.27 (1)
J-F	0 / 484	0.11 (1)
J-G	-70 / 102	0.04 (1)
M-C	-1660 / 0	0.93 (1)
G-I	-1632 / 0	0.85 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.23")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 761 (0.38")

CSI: TC=0.24/1.00 (B-C:1) , BC=0.58/1.00 (J-L:2) , WB=0.93/1.00 (C-M:1) , SSI=0.19/1.00 (L-M:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1658

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (M) (INPUT = 0.90)
JSI METAL= 0.40 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
11678
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 3985-18
STRUCTURAL

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43994	DRWG NO.
285994	T6A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Dec 12 2017 MiTek Industries, Inc. Tue Jan 16 10:45:48 2018 Page 1

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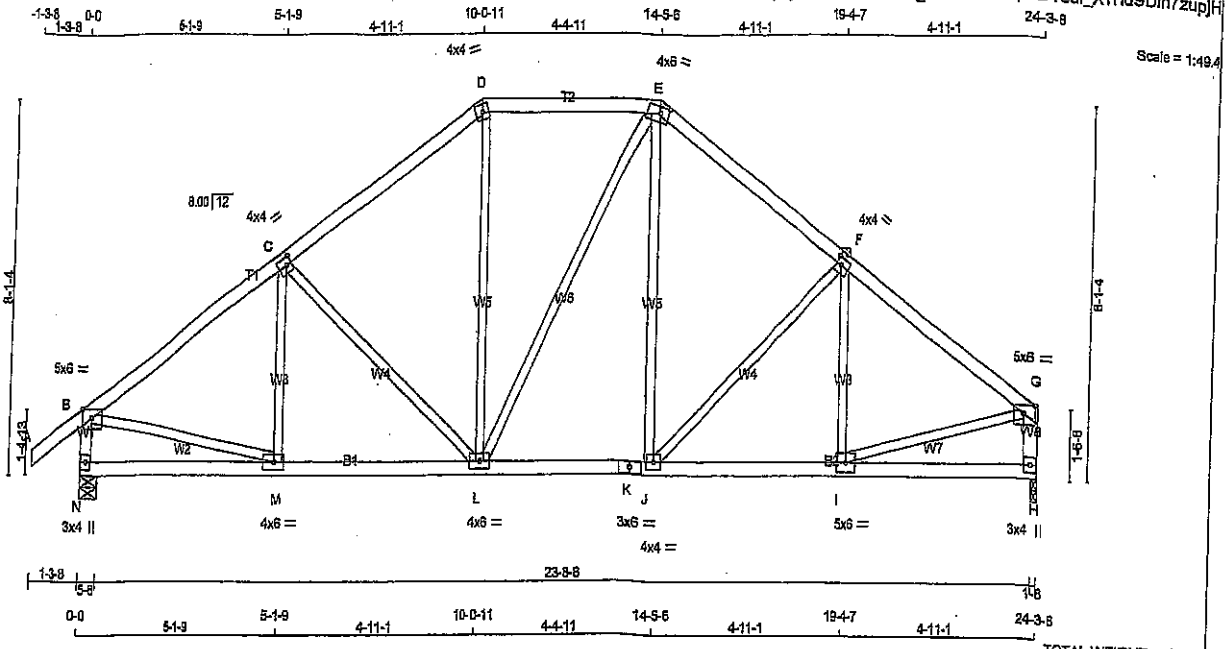


TOTAL WEIGHT = 109 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESGR
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
O	TTW-m	MT20	4.0	4.0		
E	TTWW-m	MT20	4.0	6.0	1.75	2.50
F	TMWW-t	MT20	4.0	4.0	2.00	1.50
G	TMWW-p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	5.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BB-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	4.0	6.0		
M	BMWW-t	MT20	4.0	6.0		
N	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1479	0	1479	0	0	5-8	5-8
H	1363	0	1363	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1198	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0
H	1119	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

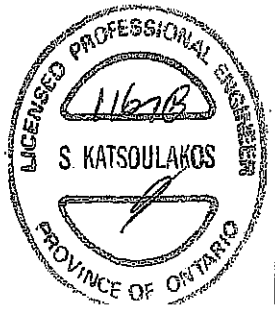
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00
B-C	-1531 / 0	-84.3	-84.3	0.31 (1)	5.00
C-D	-1271 / 0	-84.3	-84.3	0.29 (1)	5.39
D-E	-1037 / 0	-84.3	-84.3	0.22 (1)	5.91
E-F	-1282 / 0	-84.3	-84.3	0.26 (1)	5.42
F-G	-1483 / 0	-84.3	-84.3	0.29 (1)	5.08
N-B	-1420 / 0	0.0	0.0	0.15 (1)	6.64
H-G	-1307 / 0	0.0	0.0	0.14 (1)	7.06
N-M	0 / 0	-28.0	-28.0	0.18 (3)	10.00
M-L	0 / 1296	-28.0	-28.0	0.32 (2)	10.00
L-K	0 / 1030	-28.0	-28.0	0.23 (2)	10.00
K-J	0 / 1030	-28.0	-28.0	0.23 (2)	10.00
J-I	0 / 1257	-28.0	-28.0	0.31 (2)	10.00
I-H	0 / 0	-28.0	-28.0	0.17 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M-C	-115 / 147	0.04 (1)
C-L	-354 / 0	0.30 (1)
L-D	0 / 374	0.08 (2)
L-E	0 / 15	0.00 (1)
J-E	0 / 360	0.08 (2)
J-F	-310 / 0	0.26 (1)
I-F	-156 / 122	0.05 (1)
B-M	0 / 1325	0.30 (1)
I-G	0 / 1284	0.29 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.08")

CSI: TC=0.31/1.00 (B-C:1) , BC=0.32/1.00 (L-M:2) , WB=0.30/1.00 (C-L:1) , SSI=0.17/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (M) (INPUT = 0.90)
JSI METAL= 0.32 (M) (INPUT = 1.00)

DWG NO. TAM3986-13
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285994	T7A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Dec 12 2017 MiTek Industries, Inc. Tue Jan 16 10:45:49 2018 Page 1
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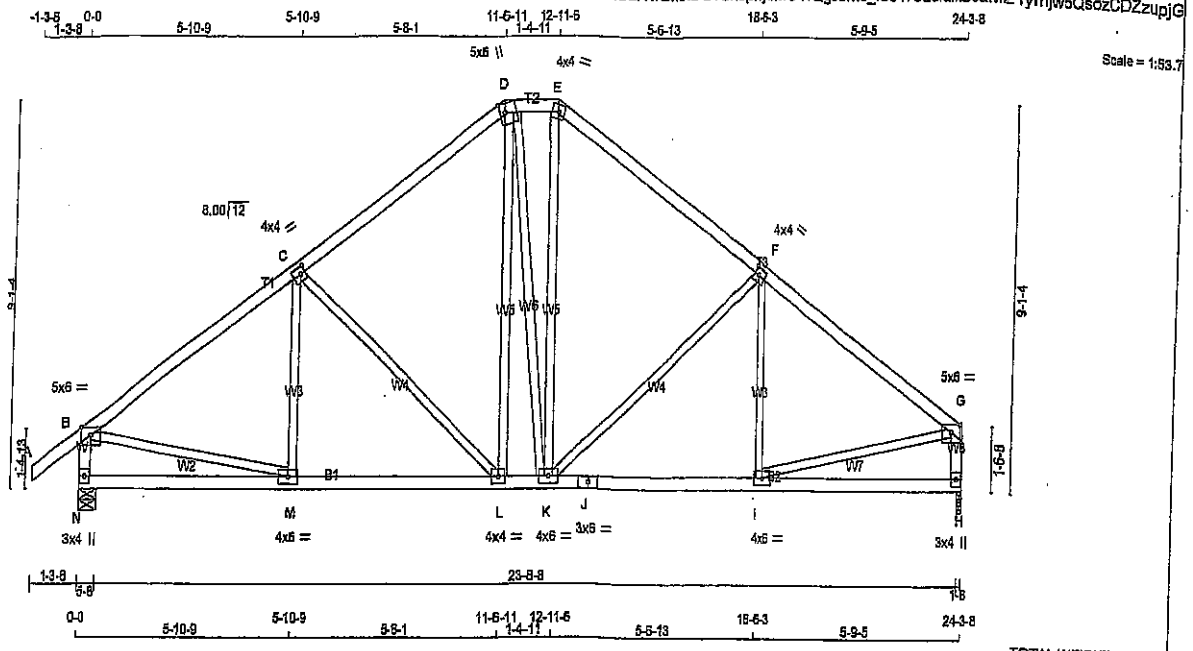


TOTAL WEIGHT = 115 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESGR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	8.0	2.50	1.50
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0	2.00	1.50
G	TMWW-p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMWW-t	MT20	4.0	6.0		
N	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
N	1479	0	1479	0	0	5-8	5-8
H	1363	0	1363	0	0	1-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1198	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0
H	1119	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

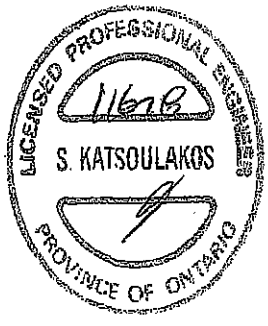
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.3	-84.3	0.11 (1)		10.00
B-C	-1533 / 0	-84.3	-84.3	0.42 (1)		4.86
C-D	-1163 / 0	-84.3	-84.3	0.36 (1)		5.44
D-E	-945 / 0	-84.3	-84.3	0.03 (1)		6.25
E-F	-1164 / 0	-84.3	-84.3	0.37 (1)		5.46
F-G	-1491 / 0	-84.3	-84.3	0.39 (1)		4.94
N-B	-1413 / 0	0.0	0.0	0.15 (1)		6.86
H-G	-1299 / 0	0.0	0.0	0.14 (1)		7.08
N-M	0 / 0	-28.0	-28.0	0.25 (3)		10.00
M-L	0 / 1302	-28.0	-28.0	0.40 (2)		10.00
L-K	0 / 943	-28.0	-28.0	0.26 (2)		10.00
K-J	0 / 1268	-28.0	-28.0	0.37 (2)		10.00
J-I	0 / 1268	-28.0	-28.0	0.37 (2)		10.00
I-H	0 / 0	-28.0	-28.0	0.23 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M-C	-50 / 214	0.05 (3)
C-L	-481 / 0	0.57 (1)
L-D	0 / 377	0.08 (1)
D-K	0 / 17	0.00 (2)
K-E	0 / 390	0.09 (1)
K-F	-433 / 0	0.51 (1)
I-F	-94 / 185	0.04 (3)
B-M	0 / 1324	0.30 (1)
I-G	0 / 1296	0.29 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(65 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.42/1.00 (B-C:1) , BC=0.40/1.00 (L-M:2) , WB=0.57/1.00 (C-L:1) , SSI=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS SEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.80)
JSI METAL= 0.33 (M) (INPUT = 1.00)

DWG NO. TAM 3987 -7B
STRUCTURAL

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285301	T8Z2	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Wed May 16 08:35:53 2018 Page 1

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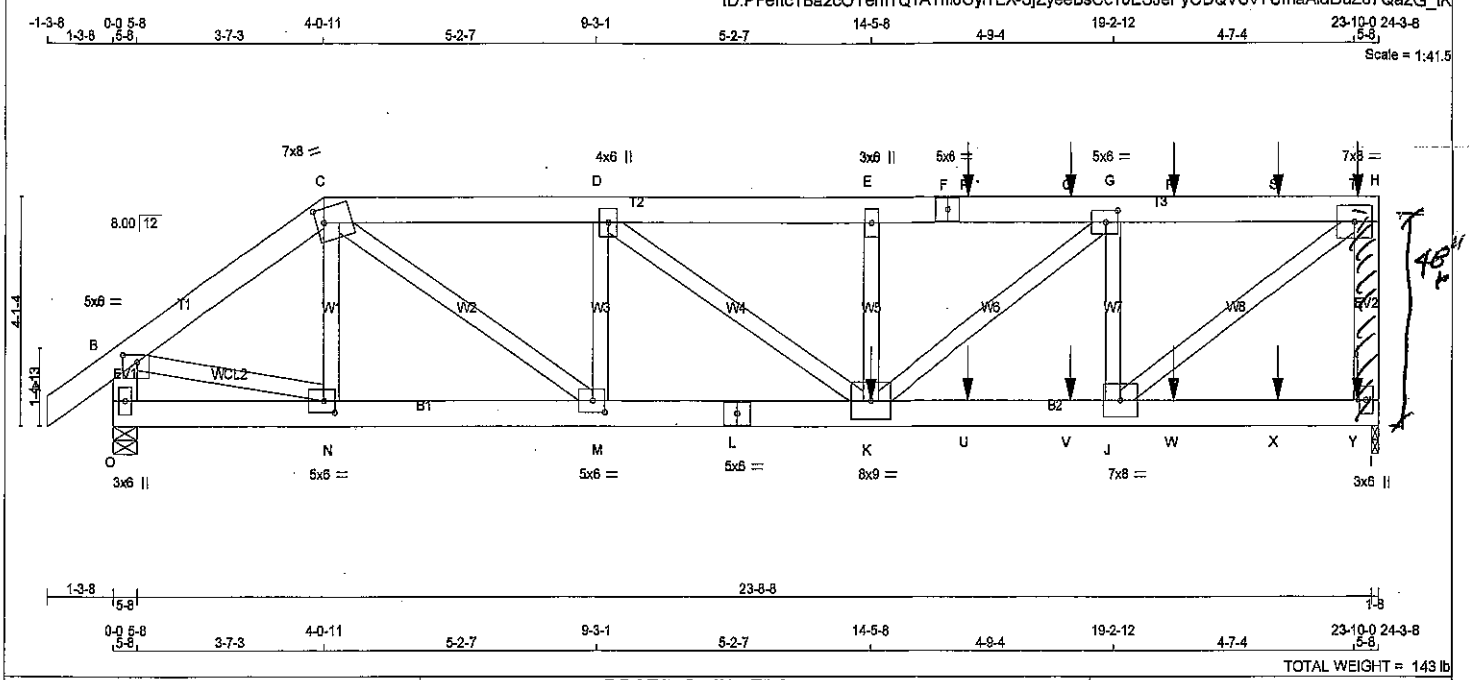

TOTAL WEIGHT = 143 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
I - H	2x6	DRY	No.2	SPF
O - B	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.25
C	TTWW-m	MT20	7.0	6.0	3.00	1.75
D	TMWW+t	MT20	4.0	6.0		
E	TMW+w	MT20	3.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMWW-t	MT20	5.0	6.0	2.50	2.75
H	TMVW-t	MT20	7.0	6.0		
I	BMV1+p	MT20	3.0	6.0		
J	BMWW-t	MT20	7.0	8.0		
K	BMWWW-t	MT20	8.0	9.0		
L	BS-t	MT20	5.0	6.0		
M	BMWW-t	MT20	5.0	6.0	2.50	2.75
N	BMWW-t	MT20	5.0	6.0	2.50	2.50
O	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 101.4 lbs FACTORED DOWN AT 16-4-12, 101.4 lbs FACTORED DOWN AT 18-4-12, 101.4 lbs FACTORED DOWN AT 20-4-12, AND 101.4 lbs FACTORED DOWN AT 22-4-12, AND 126.9 lbs FACTORED DOWN AT 23-10-12 ON TOP CHORD, AND 1454.0 lbs FACTORED DOWN AT 14-5-8, 69.9 lbs FACTORED DOWN AT 16-4-12, 69.9 lbs FACTORED DOWN AT 18-4-12, 69.9 lbs FACTORED DOWN AT 20-4-12, AND 69.9 lbs FACTORED DOWN AT 22-4-12, AND 84.7 lbs FACTORED DOWN AT 23-10-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

6) 48" VERTICAL SCAB

SCAB JOINT # I & O WITH 2 X 6 #2 SPF 24" LONG 1 SIDE(S) USING 2 ROW(S) 3 1/4" ARDOX SPIRAL/~~COMMON WIRE~~ NAILS AT 4" O/C STAGGERED (20 NAILS/SCAB). REQUIRED FOR 1 1/2" BRG PROVISION.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	2853	0	2853	0	0	1-8	1-8
O	2188	0	2188	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	2340	1303 / 0	531 / 0	0 / 0	0 / 0	507 / 0	0 / 0
O	1773	1023 / 0	378 / 0	0 / 0	0 / 0	372 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.55 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 33	-84.3	-84.3	0.07 (1)	10.00	N-C	-295 / 33	0.05 (1)
B-C	-2521 / 0	-84.3	-84.3	0.17 (1)	5.04	C-M	0 / 2343	0.41 (1)
C-D	-4011 / 0	-84.3	-84.3	0.27 (1)	4.08	M-D	-1174 / 0	0.20 (1)
D-E	-4986 / 0	-84.3	-84.3	0.31 (1)	3.67	D-K	0 / 1195	0.21 (1)
E-F	-4986 / 0	-84.3	-84.3	0.41 (1)	3.55	K-E	-417 / 0	0.07 (1)
F-P	-4986 / 0	-84.3	-84.3	0.41 (1)	3.55	K-G	0 / 2136	0.36 (1)
P-Q	-4986 / 0	-84.3	-84.3	0.41 (1)	3.55	J-G	-2079 / 0	0.36 (1)
Q-G	-4986 / 0	-84.3	-84.3	0.41 (1)	3.55	J-H	0 / 4052	0.72 (1)
G-R	-3290 / 0	-84.3	-84.3	0.33 (1)	4.35	B-N	0 / 2139	0.38 (1)
R-S	-3290 / 0	-84.3	-84.3	0.33 (1)	4.35			
S-T	-3290 / 0	-84.3	-84.3	0.33 (1)	4.35			
T-H	-3290 / 0	-84.3	-84.3	0.33 (1)	4.35			
I-H	-2720 / 0	0.0	0.0	0.40 (1)	6.29			
O-B	-2143 / 0	0.0	0.0	0.15 (1)	6.91			
O-N	0 / 0	-28.0	-28.0	0.08 (3)	10.00			
N-M	0 / 2080	-28.0	-28.0	0.31 (1)	10.00			
M-L	0 / 4011	-28.0	-28.0	0.61 (1)	10.00			
L-K	0 / 4011	-28.0	-28.0	0.61 (1)	10.00			
K-U	0 / 3290	-28.0	-28.0	0.53 (1)	10.00			
U-V	0 / 3290	-28.0	-28.0	0.53 (1)	10.00			
V-J	0 / 3290	-28.0	-28.0	0.53 (1)	10.00			
J-W	0 / 0	-28.0	-28.0	0.15 (3)	10.00			
W-X	0 / 0	-28.0	-28.0	0.15 (3)	10.00			
X-Y	0 / 0	-28.0	-28.0	0.15 (3)	10.00			
Y-I	0 / 0	-28.0	-28.0	0.15 (3)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	
K	14-5-8	-1454	-1454	---	BACK	VERT	TOTAL
P	16-4-12	-101	-101	---	BACK	VERT	TOTAL
Q	18-4-12	-101	-101	---	BACK	VERT	TOTAL
R	20-4-12	-101	-101	---	BACK	VERT	TOTAL
S	22-4-12	-101	-101	---	BACK	VERT	TOTAL
T	23-10-12	-127	-127	---	BACK	VERT	TOTAL
U	16-4-12	-40	-70	---	BACK	VERT	TOTAL
V	18-4-12	-40	-70	---	BACK	VERT	TOTAL
W	20-4-12	-40	-70	---	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER S. KATSOULAKOS PROVINCE OF ONTARIO 5/16

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.25")

CSI: TC=0.41/1.00 (E-G:1) , BC=0.61/1.00 (K-M:1) , WB=0.72/1.00 (H-J:1) , SSI=0.27/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	622	2264	1856

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.89 (L) (INPUT = 1.00)

Po 1/2

DWG NO. TAM 26864-18
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285301	T8Z2	1	1	TRUSS DESC.	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
X	22-4-12	-40	-70	—	BACK	VERT	TOTAL
Y	23-10-12	-48	-85	—	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM26964-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285994	T9	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Dec 12 2017 MiTek Industries, Inc. Tue Jan 16 10:45:49 2018 Page 1

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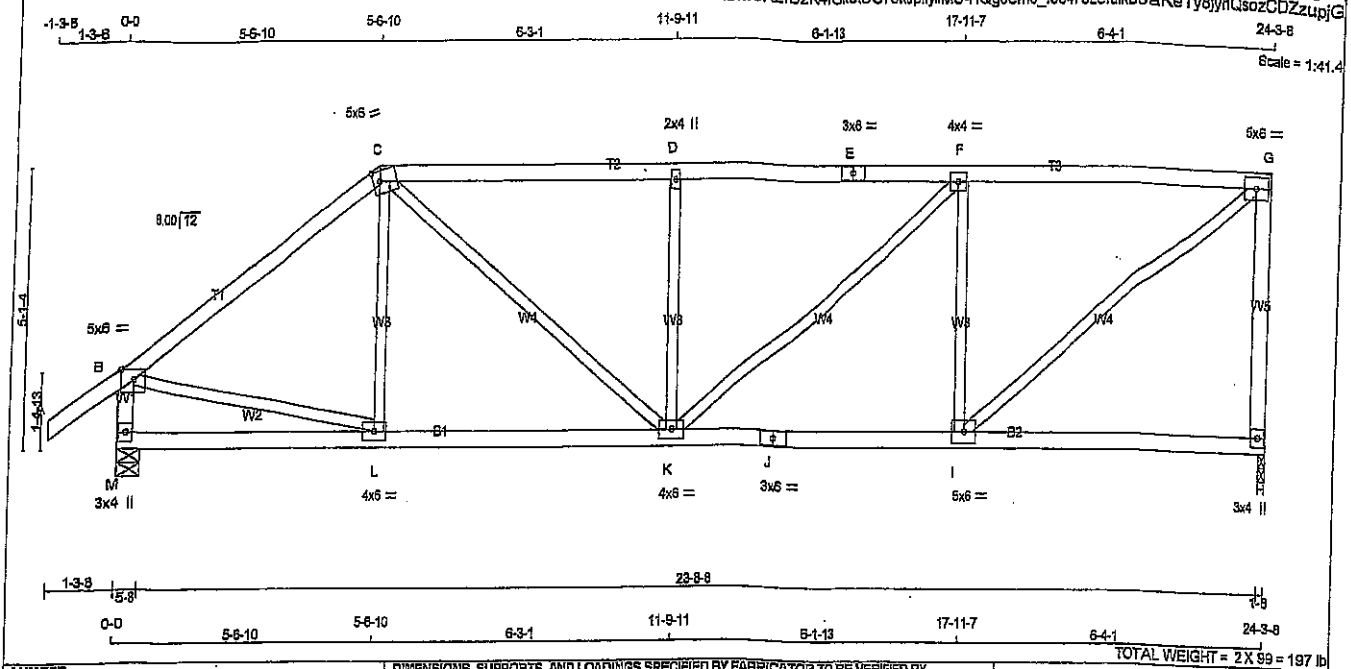

TOTAL WEIGHT = 2 X 99 = 197 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESGR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	5.0	6.0	Edge	
C	TTWW-m	MT20	5.0	6.0	2.25	1.75
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMWW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H	1363	0	1363	0	0	1-8	1-8
M	1479	0	1479	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1119	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
M	1198	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.84 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF)	CSI (LC)	MAX UNBRAC LENGTH
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	
B-C	-1493 / 0	-84.3	-84.3	0.55 (1)	4.69	
C-D	-1797 / 0	-84.3	-84.3	0.55 (1)	4.37	
D-E	-1797 / 0	-84.3	-84.3	0.56 (1)	4.34	
E-F	-1797 / 0	-84.3	-84.3	0.55 (1)	4.34	
F-G	-1416 / 0	-84.3	-84.3	0.53 (1)	4.81	
H-G	-1292 / 0	0.0	0.0	0.57 (1)	7.10	
M-B	-1417 / 0	0.0	0.0	0.15 (1)	6.85	
M-L	0 / 0	-28.0	-28.0	0.24 (3)	10.00	
L-K	0 / 1240	-28.0	-28.0	0.39 (2)	10.00	
K-J	0 / 1416	-28.0	-28.0	0.44 (2)	10.00	
J-I	0 / 1416	-28.0	-28.0	0.44 (2)	10.00	
I-H	0 / 0	-28.0	-28.0	0.27 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	CSI (LC)
L-C	-49 / 204	0.05 (3)
B-L	0 / 1283	0.28 (1)
I-G	0 / 1778	0.40 (1)
C-K	0 / 702	0.16 (1)
I-F	-877 / 0	0.34 (1)
K-D	-556 / 0	0.22 (1)
K-F	0 / 484	0.11 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 5.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.57/1.00 (G-H:1) , BC=0.44/1.00 (I-K:2) , WB=0.40/1.00 (G-I:1) , SSI=0.25/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (K) (INPUT = 0.90)
JSI METAL= 0.44 (J) (INPUT = 1.00)

DWG NO. TAM 3988-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285994	T10	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Dec 12 2017 MiTek Industries, Inc. Tue Jan 16 10:45:49 2018 Page 1
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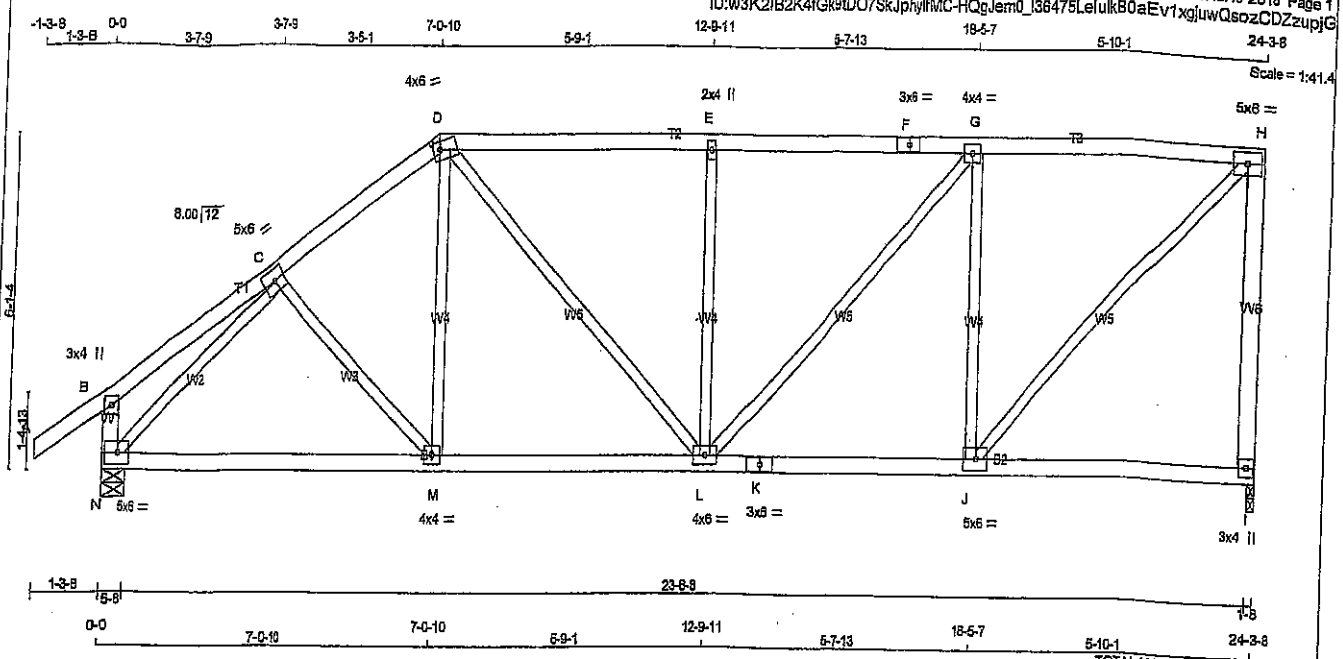

TOTAL WEIGHT = 2 X 106 = 213 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW-m	MT20	4.0	6.0	1.75	1.75
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TMVW-t	MT20	5.0	6.0		
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	5.0	6.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	4.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
I	1363	0	1363	0	0	1-8	1-8
N	1479	0	1479	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1119	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
N	1198	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.86 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	
B- C	0 / 17	-84.3	-84.3	0.15 (1)	10.00	
C- D	-1460 / 0	-84.3	-84.3	0.16 (1)	5.22	
D- E	-1473 / 0	-84.3	-84.3	0.43 (1)	4.86	
E- F	-1473 / 0	-84.3	-84.3	0.44 (1)	4.86	
F- G	-1473 / 0	-84.3	-84.3	0.44 (1)	4.86	
G- H	-1103 / 0	-84.3	-84.3	0.42 (1)	5.46	
I- H	-1298 / 0	0.0	0.0	0.94 (1)	7.08	
N- B	-233 / 0	0.0	0.0	0.02 (1)	7.81	
N- M	0 / 1196	-28.0	-28.0	0.45 (2)	10.00	
M- L	0 / 1202	-28.0	-28.0	0.47 (2)	10.00	
L- K	0 / 1103	-28.0	-28.0	0.37 (2)	10.00	
K- J	0 / 1103	-28.0	-28.0	0.37 (2)	10.00	
J- I	0 / 0	-28.0	-28.0	0.24 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- M	0 / 118	0.03 (3)
M- D	0 / 307	0.07 (2)
N- C	-1669 / 0	0.64 (1)
J- H	0 / 1557	0.35 (1)
D- L	0 / 365	0.09 (1)
J- G	-912 / 0	0.53 (1)
L- E	-520 / 0	0.30 (1)
L- G	0 / 530	0.12 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.94/1.00 (H-I:1) , BC=0.47/1.00 (L-M:2) , WB=0.64/1.00 (C-N:1) , SSI=0.23/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	BEDTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (D) (INPUT = 0.90)
JSI METAL= 0.41 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
1168
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 3909 -18
STRUCTURAL

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285994	T11	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Dec 12 2017 MiTek Industries, Inc. Tue Jan 16 10:45:48 2018 Page 1
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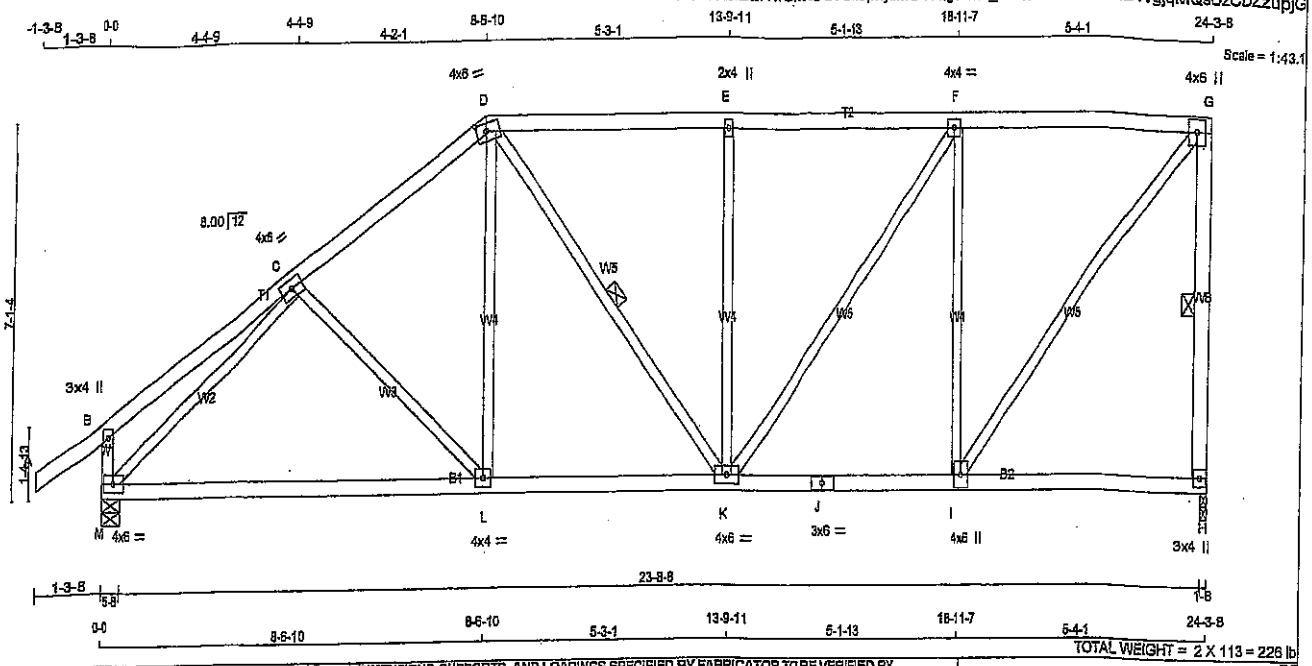


LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW-m	MT20	4.0	6.0	1.75	2.50
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVV+p	MT20	4.0	6.0		
H	SMV1+p	MT20	3.0	4.0		
I	SMWW+t	MT20	4.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	SMWWW-t	MT20	4.0	6.0		
L	SMWW-t	MT20	4.0	4.0		
M	SMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	RECRD BRG IN-SX
H	1353	0	1353	0	0	1-8	1-8
M	1479	0	1479	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1119	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
M	1198	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

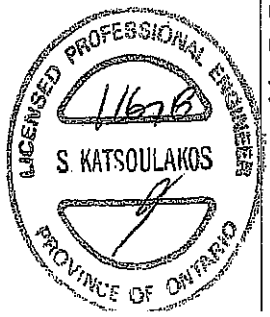
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-H, D-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX. CSI	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.3	-84.3	0.11 (1)		10.00
B-C	0 / 23	-84.3	-84.3	0.24 (1)		10.00
C-D	-1390 / 0	-84.3	-84.3	0.28 (1)		5.20
D-E	-1226 / 0	-84.3	-84.3	0.34 (1)		5.37
E-F	-1227 / 0	-84.3	-84.3	0.35 (1)		5.35
F-G	-979 / 0	-84.3	-84.3	0.34 (1)		6.07
H-G	-1305 / 0	0.0	0.0	0.30 (1)		5.66
M-B	-255 / 0	0.0	0.0	0.03 (1)		7.81
M-L	0 / 1223	-28.0	-28.0	0.59 (2)		10.00
L-K	0 / 1141	-28.0	-28.0	0.59 (2)		10.00
K-J	0 / 679	-28.0	-28.0	0.31 (2)		10.00
J-I	0 / 679	-28.0	-28.0	0.31 (2)		10.00
I-H	0 / 0	-28.0	-28.0	0.21 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-L	-117 / 85	0.07 (1)
L-D	0 / 414	0.09 (2)
M-C	-1661 / 0	0.93 (1)
I-G	0 / 1426	0.32 (1)
D-K	-6 / 141	0.03 (1)
I-F	-946 / 0	0.83 (1)
K-E	-474 / 0	0.41 (1)
K-F	0 / 576	0.13 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. O/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.23")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 779 (0.37")

CSI: TC=0.35/1.00 (E-F:1) , BC=0.59/1.00 (K-L:2) , WB=0.93/1.00 (C-M:1) , SSI=0.21/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1657	822	2284	1658

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.90 (M) (INPUT = 0.90)
JSI METAL= 0.40 (C) (INPUT = 1.00)

DWG NO. TAM 3990-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285994	T12	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Dec 12 2017 MiTek Industries, Inc. Tue Jan 16 10:45:50 2016 Page 1
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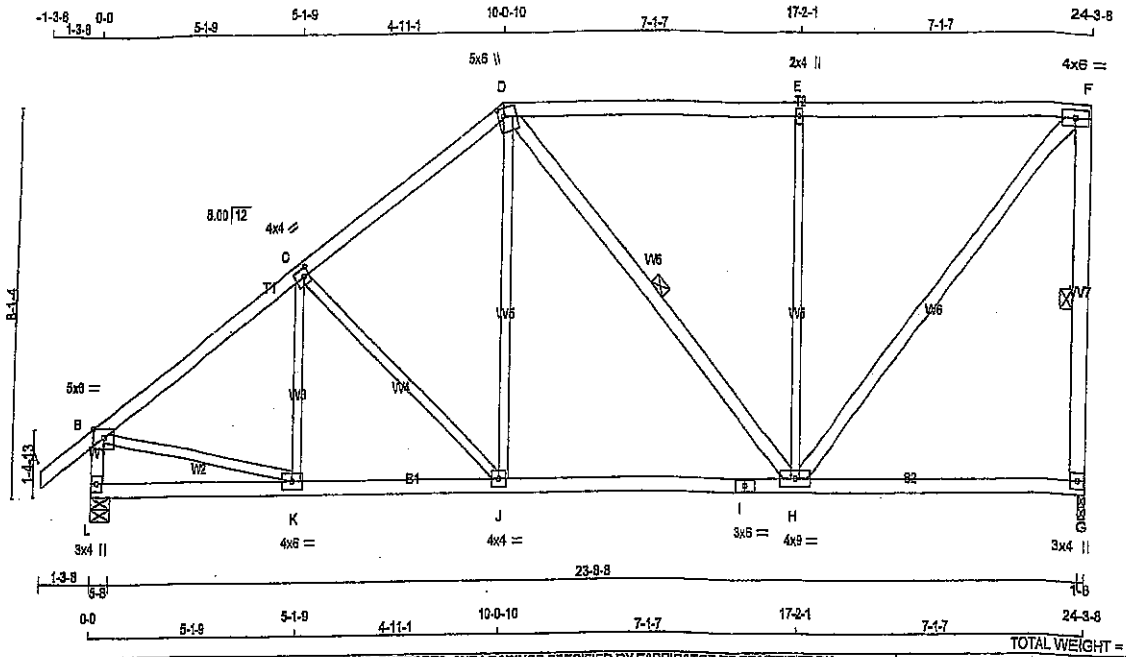

TOTAL WEIGHT = 2 X 117 = 235 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESGR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1363	0	1363	0	0	1-8	1-8
L	1479	0	1479	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1119	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
L	1198	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.81 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	K-C	-123 / 133	0.04 (1)
B-C	-1529 / 0	-84.3	-84.3	0.43 (1)	4.83	C-J	-347 / 0	0.29 (1)
C-D	-1275 / 0	-84.3	-84.3	0.40 (1)	5.20	J-D	0 / 436	0.10 (2)
D-E	-957 / 0	-84.3	-84.3	0.80 (1)	4.81	D-H	-110 / 0	0.06 (1)
E-F	-957 / 0	-84.3	-84.3	0.81 (1)	4.81	H-E	-745 / 0	0.94 (1)
G-F	-1284 / 0	0.0	0.0	0.36 (1)	5.69	H-F	0 / 1432	0.23 (1)
L-B	-1418 / 0	0.0	0.0	0.15 (1)	6.85	B-K	0 / 1323	0.30 (1)
L-K	0 / 0	-28.0	-28.0	0.16 (3)	10.00			
K-J	0 / 1295	-28.0	-28.0	0.33 (2)	10.00			
J-I	0 / 1041	-28.0	-28.0	0.49 (2)	10.00			
I-H	0 / 1041	-28.0	-28.0	0.49 (2)	10.00			
H-G	0 / 0	-28.0	-28.0	0.37 (3)	10.00			

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.81/1.00 (E-F:1) , BC=0.49/1.00 (H-J:2) , WB=0.94/1.00 (E-H:1) , SSI=0.29/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(ORY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL= 0.36 (I) (INPUT = 1.00)

DWG NO. TAM 3991-12
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
286039	T13	5	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Dec 12 2017 MiTek Industries, Inc. Tue Jan 16 10:38:29 2018 Page 1
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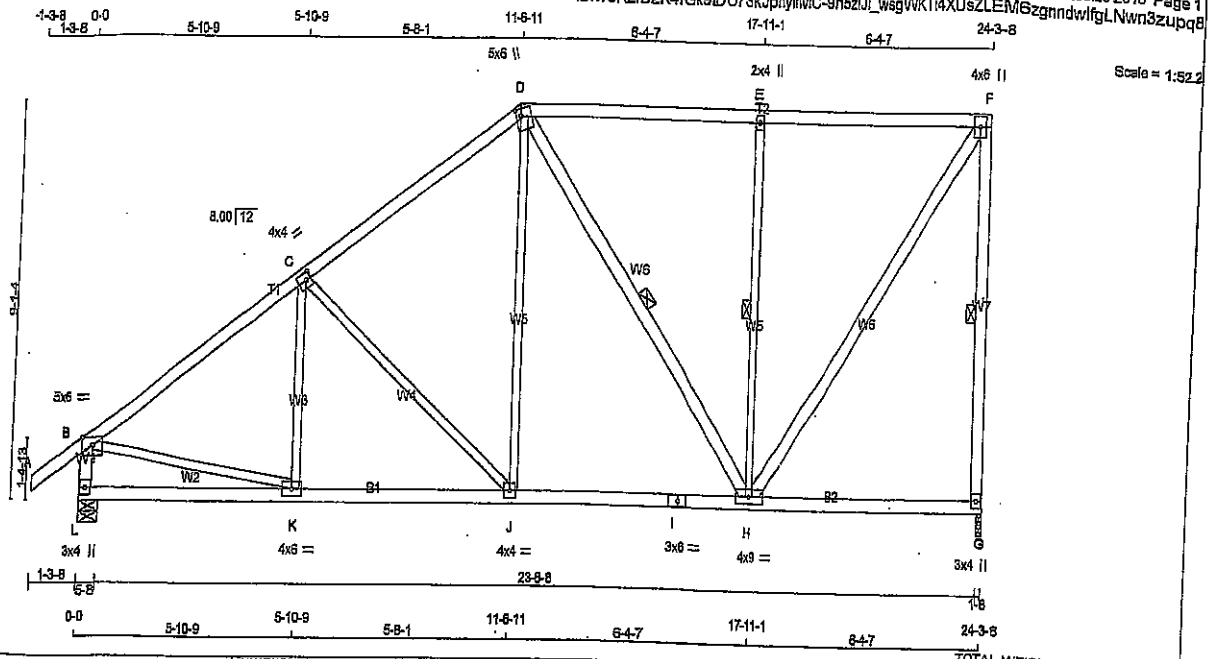


TOTAL WEIGHT = 5 X 123 = 614 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMWW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1363	0	1363	0	0	1-8	1-8
L	1479	0	1479	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1119	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
L	1198	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.81 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H, B-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	K-C	-58 / 203	0.05 (3)
B-C	-1531 / 0	-84.3	-84.3	0.59 (1)	4.61	C-J	-474 / 0	0.56 (1)
C-D	-1167 / 0	-84.3	-84.3	0.54 (1)	5.16	J-D	0 / 490	0.11 (2)
D-E	-790 / 0	-84.3	-84.3	0.61 (1)	5.73	D-H	-270 / 0	0.17 (1)
E-F	-790 / 0	-84.3	-84.3	0.62 (1)	5.73	H-E	-656 / 0	0.36 (1)
G-F	-1292 / 0	0.0	0.0	0.50 (1)	5.66	H-F	0 / 1344	0.22 (1)
L-B	-1412 / 0	0.0	0.0	0.15 (1)	6.66	B-K	0 / 1322	0.30 (1)
L-K	0 / 0	-28.0	-28.0	0.23 (3)	10.00			
K-J	0 / 1300	-28.0	-28.0	0.38 (2)	10.00			
J-I	0 / 948	-28.0	-28.0	0.40 (2)	10.00			
I-H	0 / 948	-28.0	-28.0	0.40 (2)	10.00			
H-G	0 / 0	-28.0	-28.0	0.29 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.62/1.00 (E-F:1) , BC=0.40/1.00 (H-J:2) , WB=0.56/1.00 (C-J:1) , SSI=0.28/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

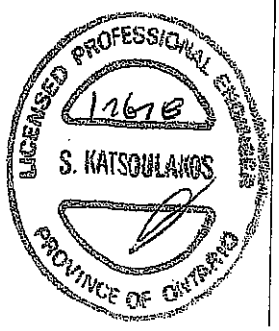
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1557	822	2284	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.32 (K) (INPUT = 1.00)



DWG NO. TAM 3993 -13
STRUCTURAL

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285426	T14	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Thu Apr 19 13:26:56 2018 Page 1

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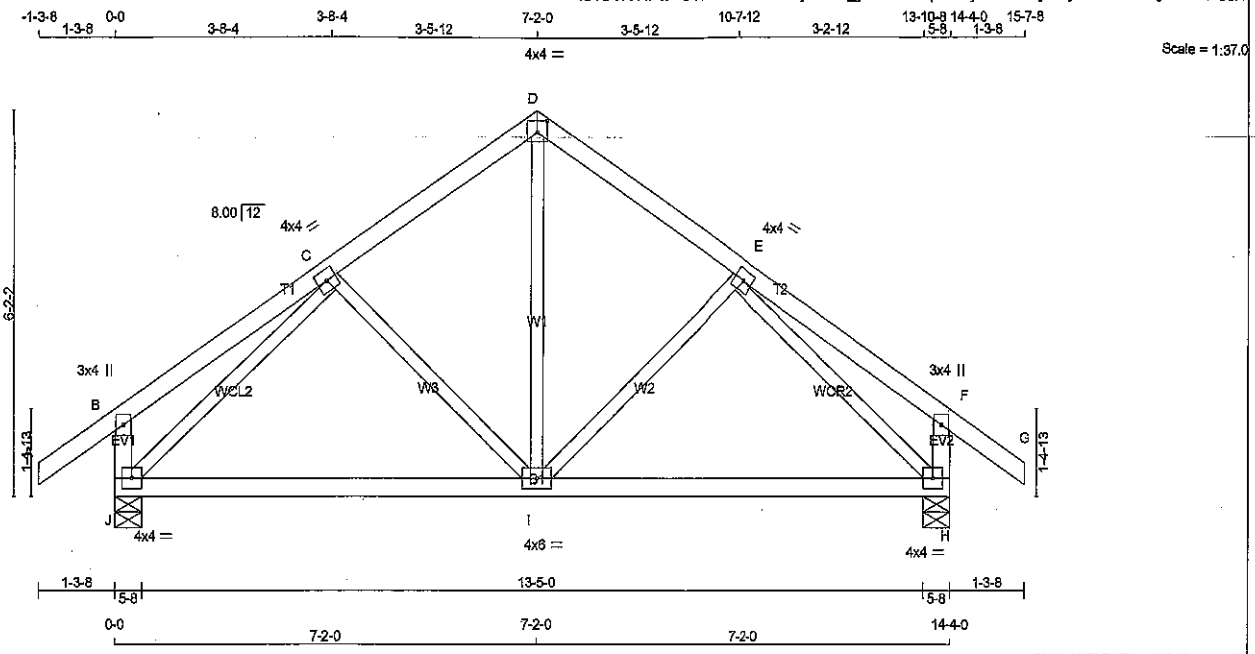

TOTAL WEIGHT = 4 X 62 = 247 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	920	0	920	0	0	5-8	5-8
H	920	0	920	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	739	437 / 0	151 / 0	0 / 0	0 / 0	152 / 0	0 / 0
H	739	437 / 0	151 / 0	0 / 0	0 / 0	152 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	I- D	0 / 460	0.10 (1)	
B- C	0 / 19	-84.3	-84.3	0.17 (1)	10.00	I- E	-152 / 44	0.06 (1)	
C- D	-645 / 0	-84.3	-84.3	0.13 (1)	6.25	C- I	-152 / 44	0.06 (1)	
D- E	-645 / 0	-84.3	-84.3	0.13 (1)	6.25	J- C	-879 / 0	0.35 (1)	
E- F	0 / 19	-84.3	-84.3	0.17 (1)	10.00	E- H	-879 / 0	0.35 (1)	
F- G	0 / 32	-84.3	-84.3	0.11 (1)	10.00				
J- B	-232 / 0	0.0	0.0	0.02 (1)	7.81				
H- F	-232 / 0	0.0	0.0	0.02 (1)	7.81				
J- I	0 / 631	-28.0	-28.0	0.49 (2)	10.00				
I- H	0 / 631	-28.0	-28.0	0.49 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.48")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.48")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.17/1.00 (E-F:1) , BC=0.49/1.00 (H-I:2) , WB=0.35/1.00 (E-H:1) , SSI=0.17/1.00 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (D) (INPUT = 0.90)
JSI METAL= 0.31 (C) (INPUT = 1.00)

DWG NO. TAM21188-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285414	T17	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Wed Aug 23 09:35:26 2017 Page 1

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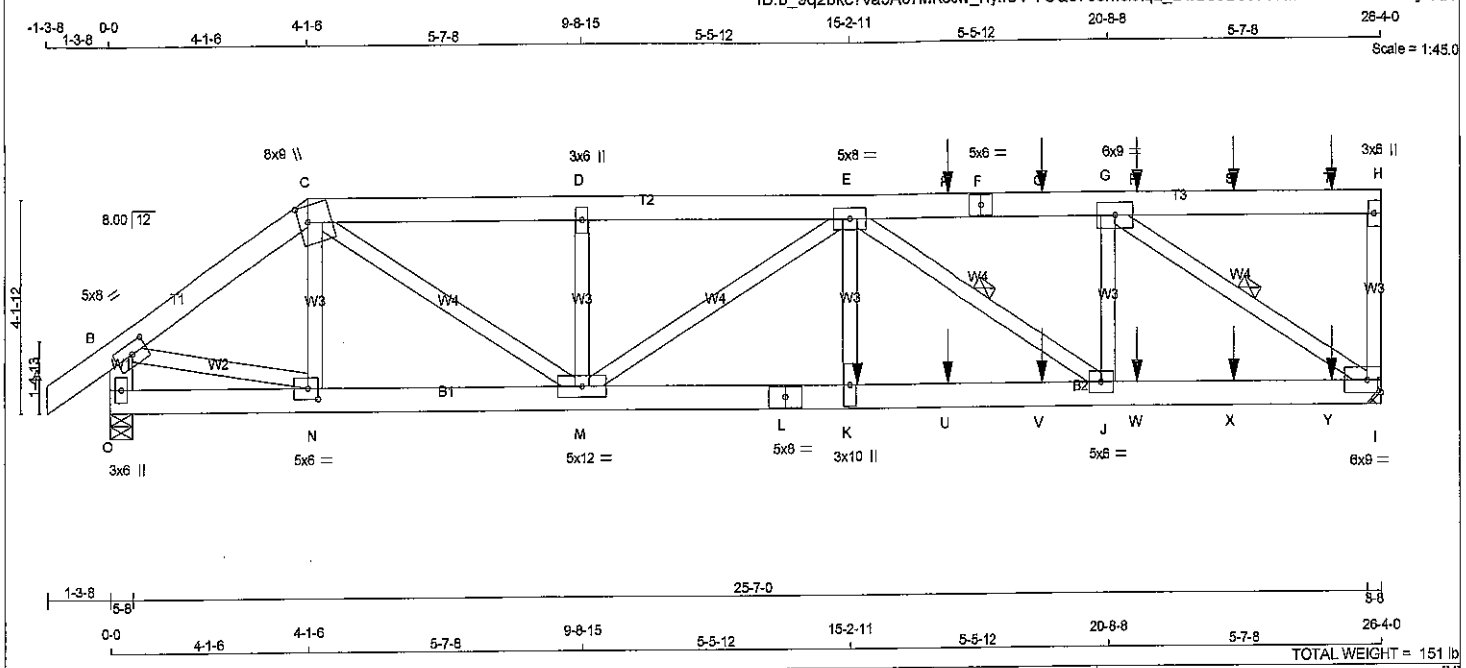


TOTAL WEIGHT = 151 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - B	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X	
B	TMVW-t	MT20	5.0	8.0	2.50	3.75	
C	TTWW+m	MT20	8.0	9.0	3.75	2.25	
D	TMW+w	MT20	3.0	6.0			
E	TMWWW-t	MT20	5.0	8.0			
F	TS-t	MT20	5.0	8.0			
G	TMWW-t	MT20	6.0	9.0			
H	TMV+p	MT20	3.0	6.0			
I	BMVW1-t	MT20	6.0	9.0			Edge
J	BMWW-t	MT20	5.0	6.0			
K	BMW+w	MT20	3.0	10.0			
L	BS-t	MT20	5.0	8.0			
M	BMWWW-t	MT20	5.0	12.0			
N	BMWW-t	MT20	5.0	6.0	2.50	2.50	
O	BMV1+p	MT20	3.0	6.0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 104.0 lbs FACTORED DOWN AT 17-3-12, 104.0 lbs FACTORED DOWN AT 19-3-12, 104.0 lbs FACTORED DOWN AT 21-3-12, AND 104.0 lbs FACTORED DOWN AT 23-3-12, AND 104.0 lbs FACTORED DOWN AT 25-3-12 ON TOP CHORD, AND 1433.7 lbs FACTORED DOWN AT 15-4-8, 71.5 lbs FACTORED DOWN AT 17-3-12, 71.5 lbs FACTORED DOWN AT 19-3-12, 71.5 lbs FACTORED DOWN AT 21-3-12, AND 71.5 lbs FACTORED DOWN AT 23-3-12, AND 71.5 lbs FACTORED DOWN AT 25-3-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
I	2893	0	2893	0	0	IN-SX	IN-SX
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-8	
O	2340	0	2340	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	2369	1325 / 0	533 / 0	0 / 0	0 / 0	511 / 0	0 / 0
O	1895	1095 / 0	402 / 0	0 / 0	0 / 0	397 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.83 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-J, G-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 33	-84.3	-84.3	0.07 (1)	10.00
B-C	-2735 / 0	-84.3	-84.3	0.18 (1)	4.86
C-D	-4486 / 0	-84.3	-84.3	0.33 (1)	3.83
D-E	-4486 / 0	-84.3	-84.3	0.30 (1)	3.66
E-P	-3770 / 0	-84.3	-84.3	0.43 (1)	3.99
P-F	-3770 / 0	-84.3	-84.3	0.43 (1)	3.99
F-Q	-3770 / 0	-84.3	-84.3	0.43 (1)	3.99
Q-G	-3770 / 0	-84.3	-84.3	0.43 (1)	3.99
G-R	0 / 0	-84.3	-84.3	0.31 (1)	10.00
R-S	0 / 0	-84.3	-84.3	0.31 (1)	10.00
S-T	0 / 0	-84.3	-84.3	0.31 (1)	10.00
T-H	0 / 0	-84.3	-84.3	0.31 (1)	10.00
I-H	-307 / 0	0.0	0.0	0.07 (1)	7.81
O-B	-2297 / 0	0.0	0.0	0.18 (1)	6.[illegible]
O-N	0 / 0	-28.0	-28.0	0.06 (2)	10.00
N-M	0 / 2258	-28.0	-28.0	0.34 (1)	10.00
M-L	0 / 5656	-28.0	-28.0	0.82 (1)	10.00
L-K	0 / 5656	-28.0	-28.0	0.82 (1)	10.00
K-U	0 / 5656	-28.0	-28.0	0.85 (1)	10.00
U-V	0 / 5656	-28.0	-28.0	0.85 (1)	10.00
V-J	0 / 5656	-28.0	-28.0	0.85 (1)	10.00
J-W	0 / 3770	-28.0	-28.0	0.59 (1)	10.00
W-X	0 / 3770	-28.0	-28.0	0.59 (1)	10.00
X-Y	0 / 3770	-28.0	-28.0	0.59 (1)	10.00
Y-I	0 / 3770	-28.0	-28.0	0.59 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N-C	-317 / 31	0.05 (1)
C-M	0 / 2660	0.47 (1)
M-D	-507 / 0	0.09 (1)
M-E	-1409 / 0	0.76 (1)
K-E	0 / 1561	0.28 (1)
E-J	-2273 / 0	0.49 (1)
J-G	0 / 1584	0.28 (1)
G-I	-4507 / 0	0.97 (1)
B-N	0 / 2320	0.41 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
K	15-4-8	-1434	-1434	—	FRONT	VERT	TOTAL
P	17-3-12	-104	-104	—	FRONT	VERT	TOTAL
Q	19-3-12	-104	-104	—	FRONT	VERT	TOTAL
R	21-3-12	-104	-104	—	FRONT	VERT	TOTAL
S	23-3-12	-104	-104	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.88")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.20")
ALLOWABLE DEFL.(TL)= L/360 (0.88")
CALCULATED VERT. DEFL.(TL) = L/ 983 (0.32")

CSI: TC=0.43 (E-G:1) , BC=0.85 (J-K:1) , WB=0.97 (G-I:1) , SSI=0.32 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

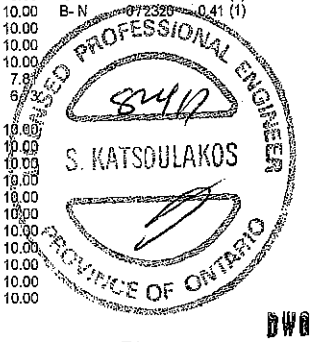
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (N) (INPUT = 0.90)
JSI METAL= 0.93 (L) (INPUT = 1.00)



DWG NO. T17M 4/08/17
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285414	T17	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
T	25-3-12	-104	-104	—	FRONT	VERT	TOTAL
U	17-3-12	-41	-71	—	FRONT	VERT	TOTAL
V	19-3-12	-41	-71	—	FRONT	VERT	TOTAL
W	21-3-12	-41	-71	—	FRONT	VERT	TOTAL
X	23-3-12	-41	-71	—	FRONT	VERT	TOTAL
Y	25-3-12	-41	-71	—	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
8/24/17
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. T17 4/8 28 17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285414	T18	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Wed Aug 23 09:35:26 2017 Page 1

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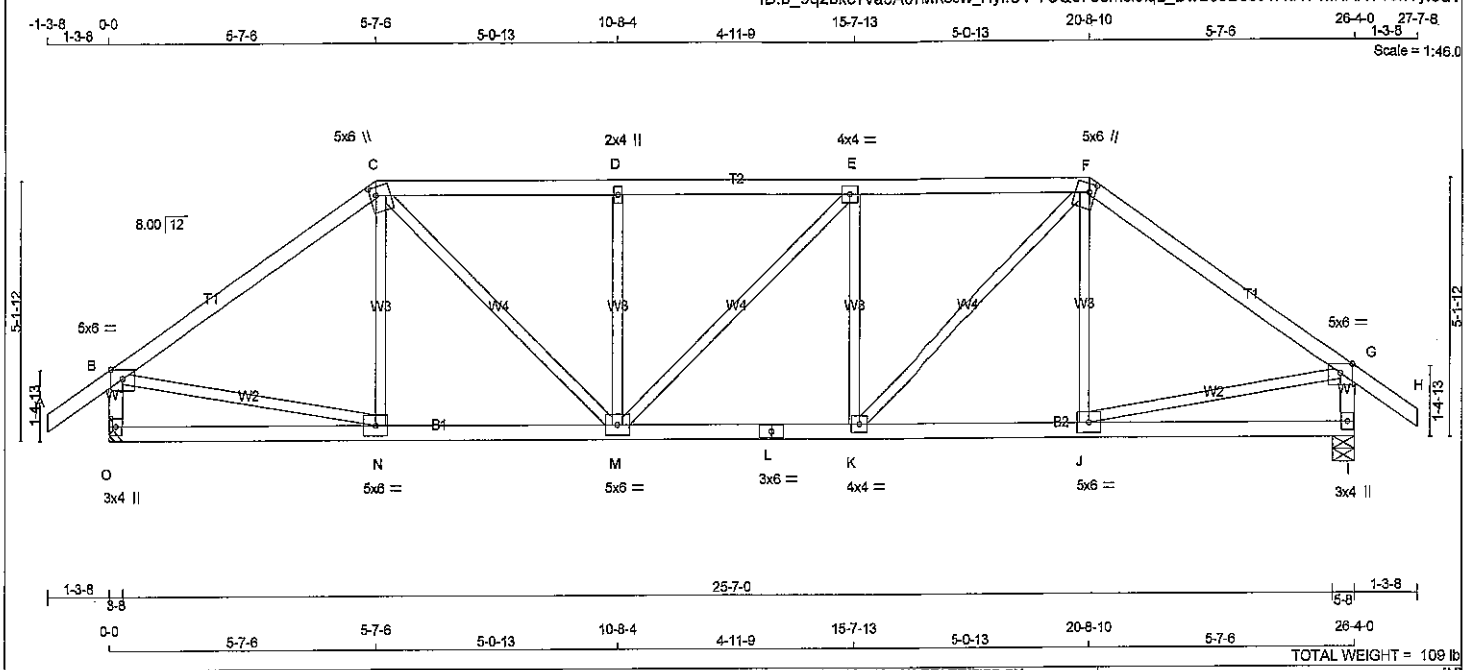

TOTAL WEIGHT = 109 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.00	1.50
G	TMVW-p	MT20	5.0	6.0	Edge	
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	5.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	5.0	6.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
O	1594	0	1594	0	0	HANGER BY OTHERS IN-SX	IN-SX
						MIN. SEAT SIZE: 3-8	
I	1594	0	1594	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1292	744 / 0	277 / 0	0 / 0	0 / 0	272 / 0	0 / 0
I	1292	744 / 0	277 / 0	0 / 0	0 / 0	272 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.43 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	
B- C	-1649 / 0	-84.3	-84.3	0.58 (1)	4.48	
C- D	-1982 / 0	-84.3	-84.3	0.36 (1)	4.44	
D- E	-1982 / 0	-84.3	-84.3	0.36 (1)	4.43	
E- F	-1984 / 0	-84.3	-84.3	0.36 (1)	4.43	
F- G	-1649 / 0	-84.3	-84.3	0.58 (1)	4.48	
G- H	0 / 32	-84.3	-84.3	0.11 (1)	10.00	
O- B	-1528 / 0	0.0	0.0	0.16 (1)	6.65	
I- G	-1528 / 0	0.0	0.0	0.16 (1)	6.65	
O- N	0 / 0	-26.0	-28.0	0.21 (3)	10.00	
N- M	0 / 1369	-28.0	-28.0	0.37 (2)	10.00	
M- L	0 / 1984	-26.0	-28.0	0.36 (1)	10.00	
L- K	0 / 1984	-26.0	-28.0	0.36 (1)	10.00	
K- J	0 / 1368	-26.0	-28.0	0.37 (2)	10.00	
J- I	0 / 0	-26.0	-28.0	0.21 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- C	-85 / 168	0.04 (3)
C- M	0 / 848	0.19 (1)
M- D	-455 / 0	0.18 (1)
M- E	-2 / 0	0.00 (1)
K- E	-456 / 0	0.18 (1)
K- F	0 / 851	0.19 (1)
J- F	-86 / 168	0.04 (3)
B- N	0 / 1395	0.31 (1)
J- G	0 / 1395	0.31 (1)

DWG NO. T.A.M 4/B29/17
STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.88")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.88")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.58 (B-C:1) , BC=0.36 (K-M:1) , WB=0.31 (B-N:1) , SSI=0.20 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.57 (L) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285414	T19	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Wed Aug 23 09:35:26 2017 Page 1

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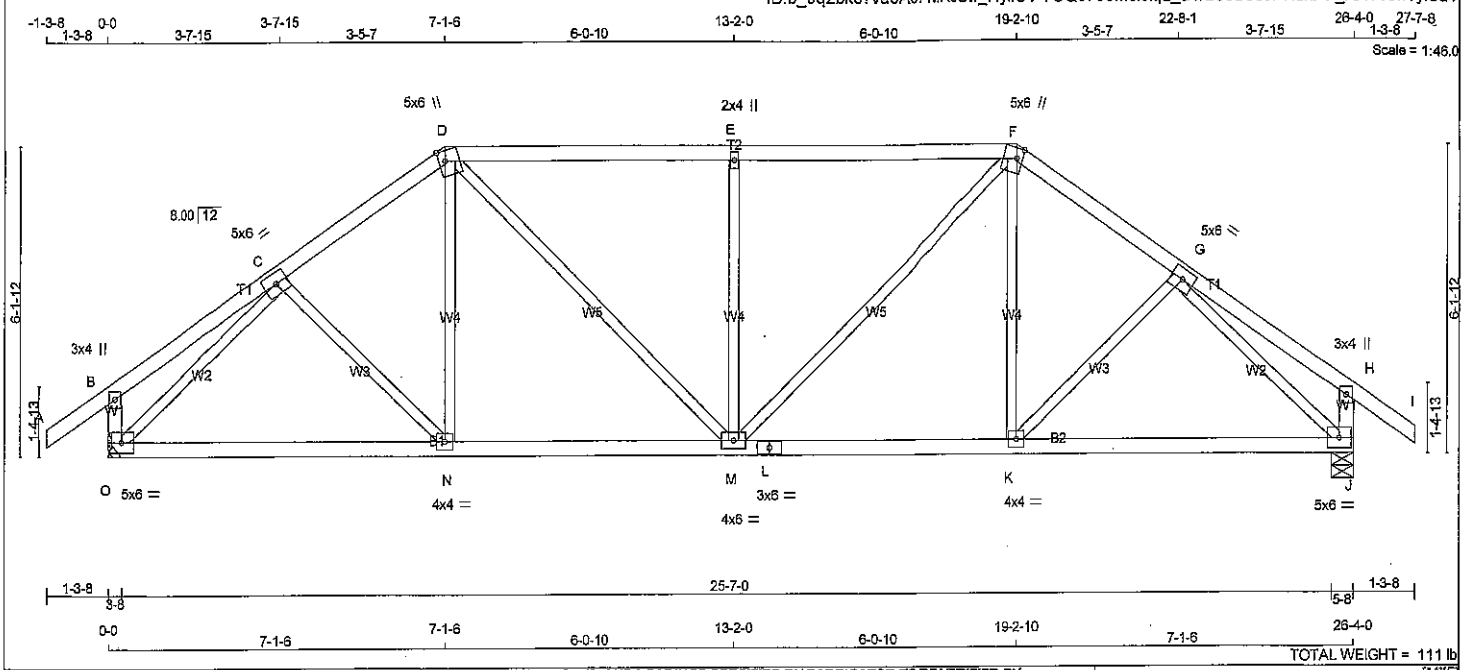


TOTAL WEIGHT = 111 lb

[MiTek]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.50	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.50	1.50
G	TMWW-t	MT20	5.0	6.0		
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	5.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
O	1594	0	1594	0	0	IN-SX HANGER BY OTHERS MIN. SEAT SIZE: 3-8	IN-SX
J	1594	0	1594	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1292	744 / 0	277 / 0	0 / 0	0 / 0	272 / 0	0 / 0
J	1292	744 / 0	277 / 0	0 / 0	0 / 0	272 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

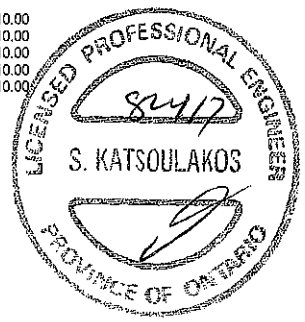
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	
B-C	0 / 17	-84.3	-84.3	0.15 (1)	10.00	
C-D	-1626 / 0	-84.3	-84.3	0.15 (1)	5.07	
D-E	-1733 / 0	-84.3	-84.3	0.44 (1)	4.58	
E-F	-1733 / 0	-84.3	-84.3	0.44 (1)	4.58	
F-G	-1626 / 0	-84.3	-84.3	0.15 (1)	5.07	
G-H	0 / 17	-84.3	-84.3	0.15 (1)	10.00	
H-I	0 / 32	-84.3	-84.3	0.11 (1)	10.00	
O-B	-234 / 0	0.0	0.0	0.02 (1)	7.81	
J-H	-234 / 0	0.0	0.0	0.02 (1)	7.81	
O-N	0 / 1316	-28.0	-28.0	0.48 (2)	10.00	
N-M	0 / 1340	-28.0	-28.0	0.50 (2)	10.00	
M-L	0 / 1340	-28.0	-28.0	0.50 (2)	10.00	
L-K	0 / 1340	-28.0	-28.0	0.50 (2)	10.00	
K-J	0 / 1316	-28.0	-28.0	0.48 (2)	10.00	

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-N	0 / 132	0.03 (3)
N-D	0 / 304	0.07 (2)
D-M	0 / 547	0.12 (1)
M-E	-624 / 0	0.37 (1)
M-F	0 / 547	0.12 (1)
K-F	0 / 304	0.07 (2)
K-G	0 / 132	0.03 (3)
O-C	-1834 / 0	0.72 (1)
G-J	-1834 / 0	0.72 (1)



DWG NO. TAM 4/B30 17

STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.88")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.88")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.44 (D-E:1) , BC=0.50 (M-N:2) , WB=0.72 (C-O:1) , SSI=0.25 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (O) (INPUT = 0.90)
JSI METAL= 0.45 (G) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285415	T20A	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Dec 12 2017 MiTek Industries, Inc. Tue Feb 6 11:09:13 2018 Page 1

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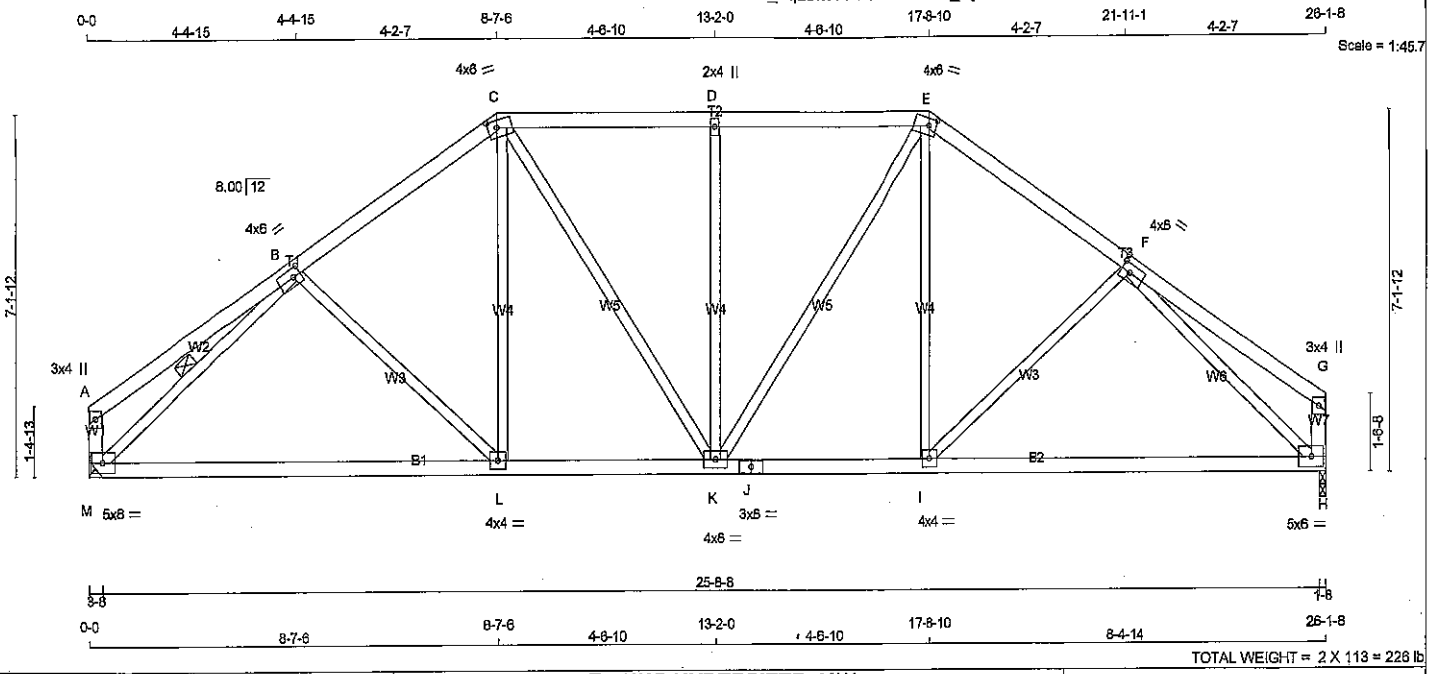

TOTAL WEIGHT = 2 X 113 = 226 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - A	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	6.0	2.00	2.00
C	TTWW-m	MT20	4.0	6.0	1.75	2.25
D	TMW+w	MT20	2.0	4.0		
E	TTWW-m	MT20	4.0	6.0	1.75	2.00
F	TMWW-t	MT20	4.0	6.0	2.00	2.25
G	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	5.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
M	1466	0	1466	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	
H	1466	0	1466	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1204	668 / 0	274 / 0	0 / 0	0 / 0	261 / 0	0 / 0
H	1204	668 / 0	274 / 0	0 / 0	0 / 0	261 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.09 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF B-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 23	-84.3	-84.3	0.24 (1)	10.00
B-C	-1543 / 0	-84.3	-84.3	0.22 (1)	5.09
C-D	-1422 / 0	-84.3	-84.3	0.24 (1)	5.22
D-E	-1422 / 0	-84.3	-84.3	0.24 (1)	5.22
E-F	-1523 / 0	-84.3	-84.3	0.21 (1)	5.13
F-G	0 / 23	-84.3	-84.3	0.23 (1)	10.00
M-A	-140 / 0	0.0	0.0	0.01 (1)	7.81
H-G	-132 / 0	0.0	0.0	0.01 (1)	7.81
M-L	0 / 1336	-28.0	-28.0	0.62 (2)	10.00
L-K	0 / 1268	-28.0	-28.0	0.63 (2)	10.00
K-J	0 / 1251	-28.0	-28.0	0.61 (2)	10.00
J-I	0 / 1251	-28.0	-28.0	0.61 (2)	10.00
I-H	0 / 1283	-28.0	-28.0	0.60 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B-L	-97 / 97	0.06 (1)
L-C	0 / 421	0.09 (2)
C-K	0 / 278	0.06 (1)
K-D	-464 / 0	0.41 (1)
K-E	0 / 309	0.07 (1)
I-E	0 / 391	0.09 (2)
I-F	-48 / 115	0.03 (1)
M-B	-1813 / 0	0.44 (1)
F-H	-1752 / 0	0.95 (1)

[M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.87")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.23")
ALLOWABLE DEFL.(TL)= L/360 (0.87")
CALCULATED VERT. DEFL.(TL) = L/ 830 (0.36")

CSI: TC=0.24/1.00 (A-B:1) , BC=0.63/1.00 (K-L:2) , WB=0.95/1.00 (F-H:1) , SSI=0.20/1.00 (L-M:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.86 (F) (INPUT = 0.90)
SI METAL= 0.44 (B) (INPUT = 1.00)

DWG NO. TAM 8099 -R1
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285415	T21A	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Wed May 16 15:37:13 2018 Page 1

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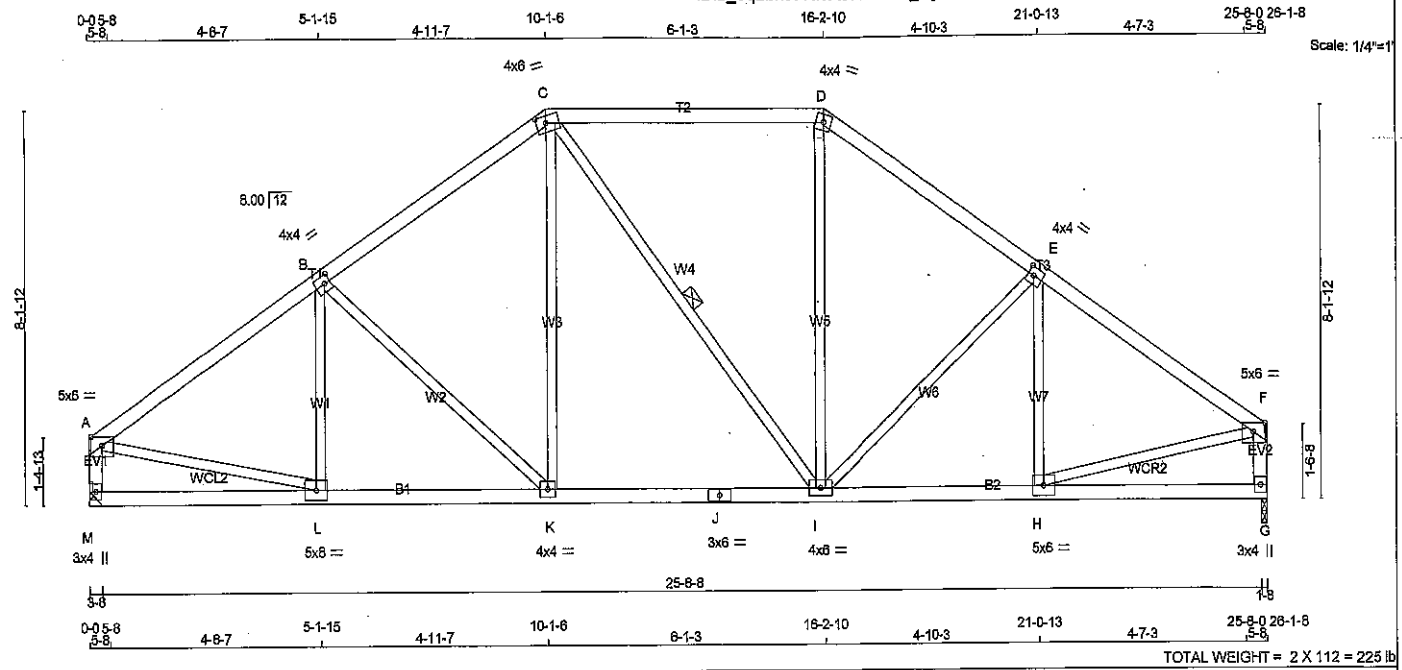

TOTAL WEIGHT = 2 X 112 = 225 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
M - A	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	6.0	Edge	
B	TMWW-t	MT20	4.0	4.0	2.00	1.50
C	TTWW-m	MT20	4.0	6.0	1.75	2.50
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.50
F	TMVW-p	MT20	5.0	6.0	Edge	
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	5.0	6.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT	REQRD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	BRG
M	1466	0	1466	0	0	IN-SX HANGER BY OTHERS MIN. SEAT SIZE: 3-8	IN-SX
G	1466	0	1466	0	0	1-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1204	668 / 0	274 / 0	0 / 0	0 / 0	261 / 0	0 / 0
G	1204	668 / 0	274 / 0	0 / 0	0 / 0	261 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.82 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO									
A- B	-1670 / 0	-84.3	-84.3	0.32 (1)	4.62	L- B	-143 / 126	0.05 (1)	
B- C	-1430 / 0	-84.3	-84.3	0.30 (1)	5.14	B- K	-333 / 0	0.28 (1)	
C- D	-1161 / 0	-84.3	-84.3	0.42 (1)	5.37	K- C	0 / 420	0.09 (2)	
D- E	-1419 / 0	-84.3	-84.3	0.29 (1)	5.17	C- I	-14 / 0	0.01 (1)	
E- F	-1617 / 0	-84.3	-84.3	0.30 (1)	4.90	I- D	0 / 409	0.09 (2)	
M- A	-1405 / 0	0.0	0.0	0.14 (1)	6.67	I- E	-289 / 0	0.24 (1)	
G- F	-1408 / 0	0.0	0.0	0.15 (1)	6.67	H- E	-182 / 101	0.06 (1)	
						A- L	0 / 1443	0.32 (1)	
M- L	0 / 0	-26.0	-26.0	0.16 (3)	10.00	H- F	0 / 1407	0.32 (1)	
L- K	0 / 1413	-26.0	-26.0	0.33 (2)	10.00				
K- J	0 / 1169	-26.0	-26.0	0.31 (2)	10.00				
J- I	0 / 1169	-26.0	-26.0	0.31 (2)	10.00				
I- H	0 / 1368	-26.0	-26.0	0.33 (2)	10.00				
H- G	0 / 0	-26.0	-26.0	0.16 (3)	10.00				

[M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.42/1.00 (C-D:1) , BC=0.33/1.00 (K-L:2) , WB=0.32/1.00 (A-L:1) , SSI=0.20/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	818	354	1667	622	2264	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (A) (INPUT = 0.90)
JSI METAL= 0.39 (J) (INPUT = 1.00)

DWG NO. TAM 2-6666-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285415	T22A	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Wed May 16 15:37:13 2018 Page 1
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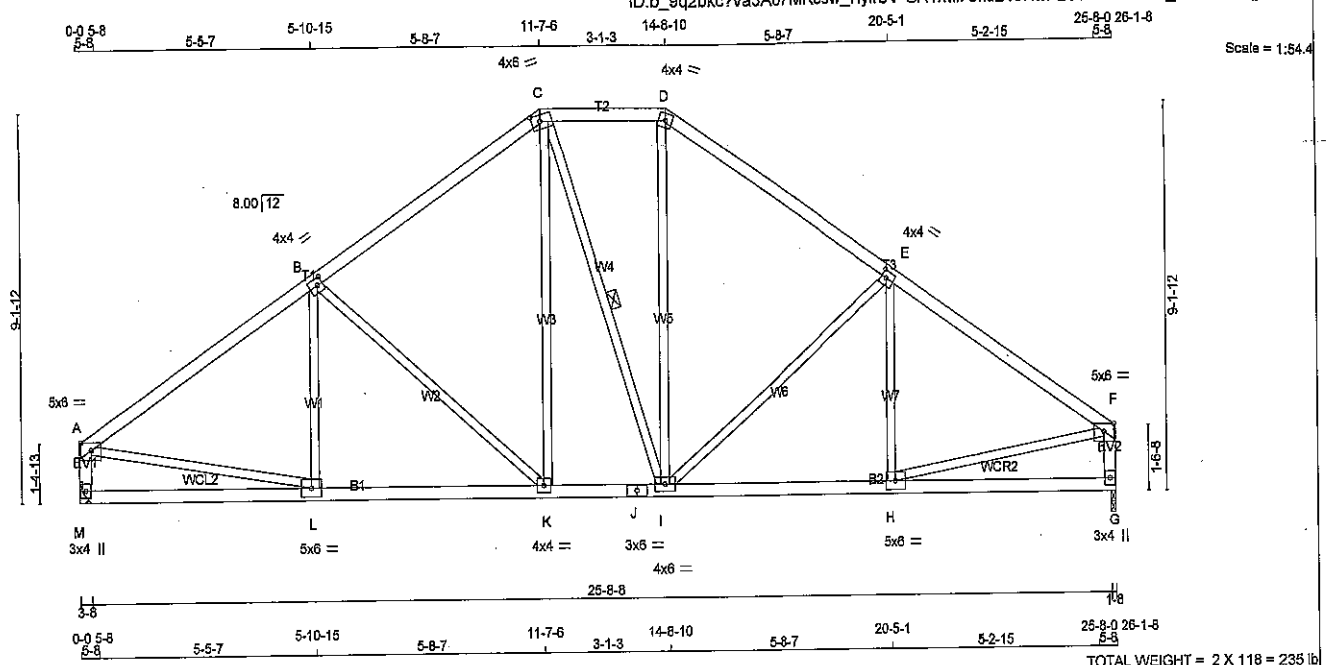


TOTAL WEIGHT = 2 X 118 = 235 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
M - A	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	6.0	Edge	
B	TMWW-t	MT20	4.0	4.0	2.00	1.50
C	TTWW-m	MT20	4.0	6.0	1.75	2.50
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.50
F	TMVW-p	MT20	5.0	6.0	Edge	
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	5.0	6.0		
I	BMWWW-t	MT20	4.0	8.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1466	0	1466	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	
G	1466	0	1466	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1204	668 / 0	274 / 0	0 / 0	0 / 0	261 / 0	0 / 0
G	1204	668 / 0	274 / 0	0 / 0	0 / 0	261 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.68 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

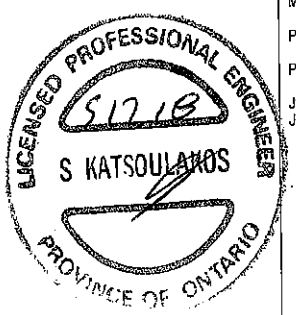
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1678 / 0	-84.3	-84.3	0.43 (1)	4.68
B- C	-1321 / 0	-84.3	-84.3	0.40 (1)	5.17
C- D	-1073 / 0	-84.3	-84.3	0.12 (1)	5.97
D- E	-1317 / 0	-84.3	-84.3	0.38 (1)	5.19
E- F	-1633 / 0	-84.3	-84.3	0.40 (1)	4.76
M- A	-1400 / 0	0.0	0.0	0.14 (1)	6.88
G- F	-1402 / 0	0.0	0.0	0.15 (1)	6.88
M- L	0 / 0	-28.0	-28.0	0.25 (3)	10.00
L- K	0 / 1423	-28.0	-28.0	0.41 (2)	10.00
K- J	0 / 1075	-28.0	-28.0	0.25 (2)	10.00
J- I	0 / 1075	-28.0	-28.0	0.25 (2)	10.00
I- H	0 / 1385	-28.0	-28.0	0.39 (2)	10.00
H- G	0 / 0	-28.0	-28.0	0.24 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- B	-70 / 207	0.05 (3)
B- K	-466 / 0	0.56 (1)
K- C	0 / 413	0.09 (1)
C- I	-8 / 0	0.00 (1)
I- D	0 / 402	0.09 (2)
I- E	-419 / 0	0.50 (1)
H- E	-113 / 179	0.05 (1)
A- L	0 / 1446	0.33 (1)
H- F	0 / 1416	0.32 (1)



[M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.87")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.87")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.43/1.00 (A-B:1) , BC=0.41/1.00 (K-L:2) , WB=0.56/1.00 (B-K:1) , SSI=0.20/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1567	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (A) (INPUT = 0.90)
JSI METAL= 0.36 (L) (INPUT = 1.00)

DWG NO. TAM26687 -18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285415	T23A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Wed May 16 15:37:13 2018 Page 1
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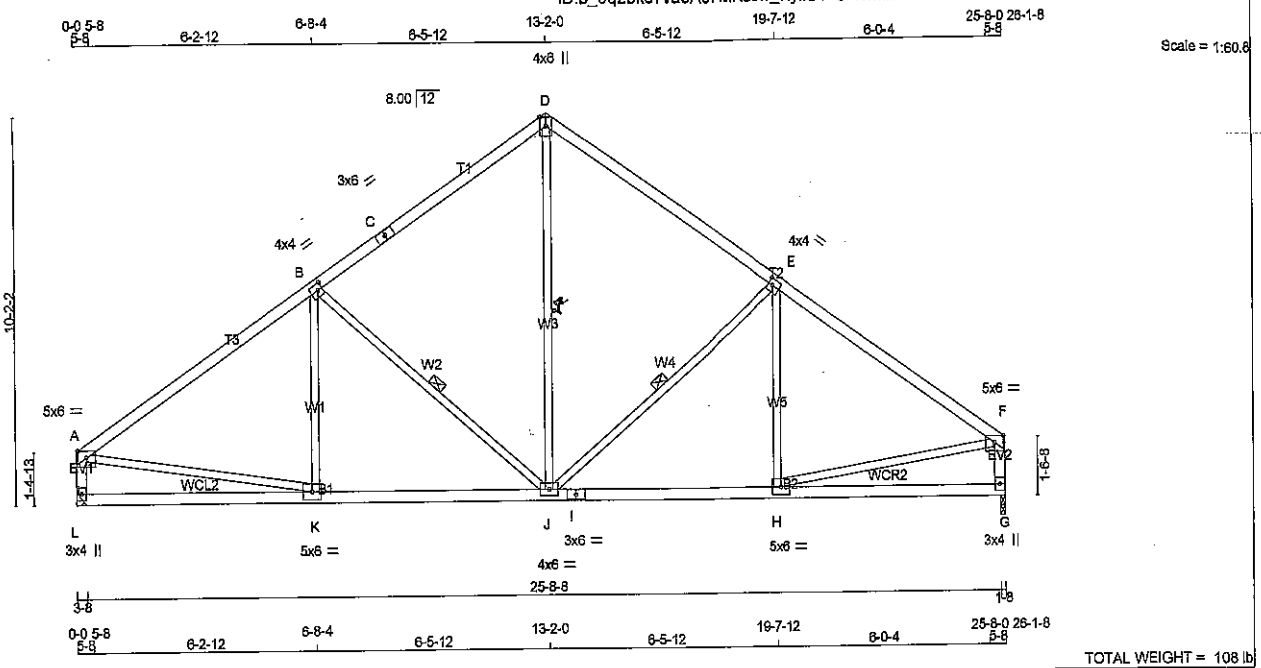


TOTAL WEIGHT = 108 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
L - A	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	6.0	Edge	
B	TMWW-t	MT20	4.0	4.0	2.00	1.50
C	TS-t	MT20	3.0	6.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	4.0	2.00	1.50
F	TMVW-p	MT20	5.0	6.0	Edge	
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	5.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
L	1466	0	1466	0	0	IN-SX	IN-SX
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-8	
G	1466	0	1466	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1204	668 / 0	274 / 0	0 / 0	0 / 0	261 / 0	0 / 0
G	1204	668 / 0	274 / 0	0 / 0	0 / 0	261 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.49 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

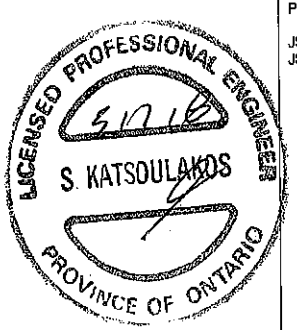
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF B-J, E-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	-1666 / 0	-84.3	-84.3	0.57 (1)	4.49	K-B	-23 / 253	0.06 (3)
B-C	-1216 / 0	-84.3	-84.3	0.51 (1)	5.15	B-J	-576 / 0	0.29 (1)
C-D	-1216 / 0	-84.3	-84.3	0.51 (1)	5.15	J-D	0 / 896	0.20 (1)
D-E	-1215 / 0	-84.3	-84.3	0.50 (1)	5.17	J-E	-536 / 0	0.27 (1)
E-F	-1631 / 0	-84.3	-84.3	0.54 (1)	4.56	H-E	-56 / 230	0.05 (3)
L-A	-1390 / 0	0.0	0.0	0.14 (1)	6.90	A-K	0 / 1437	0.32 (1)
G-F	-1393 / 0	0.0	0.0	0.15 (1)	6.90	H-F	0 / 1412	0.32 (1)
L-K	0 / 0	-28.0	-28.0	0.30 (3)	10.00			
K-J	0 / 1416	-28.0	-28.0	0.46 (2)	10.00			
J-I	0 / 1388	-28.0	-28.0	0.45 (2)	10.00			
I-H	0 / 1388	-28.0	-28.0	0.45 (2)	10.00			
H-G	0 / 0	-28.0	-28.0	0.29 (3)	10.00			



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DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.87")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.87")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.57/1.00 (A-B:1) , BC=0.46/1.00 (J-K:2) , WB=0.32/1.00 (A-K:1) , SSI=0.23/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (A) (INPUT = 0.90)
JSI METAL= 0.41 (I) (INPUT = 1.00)

DWG NO. TAM 260888-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285414	T24	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Wed Aug 23 09:35:29 2017 Page 1
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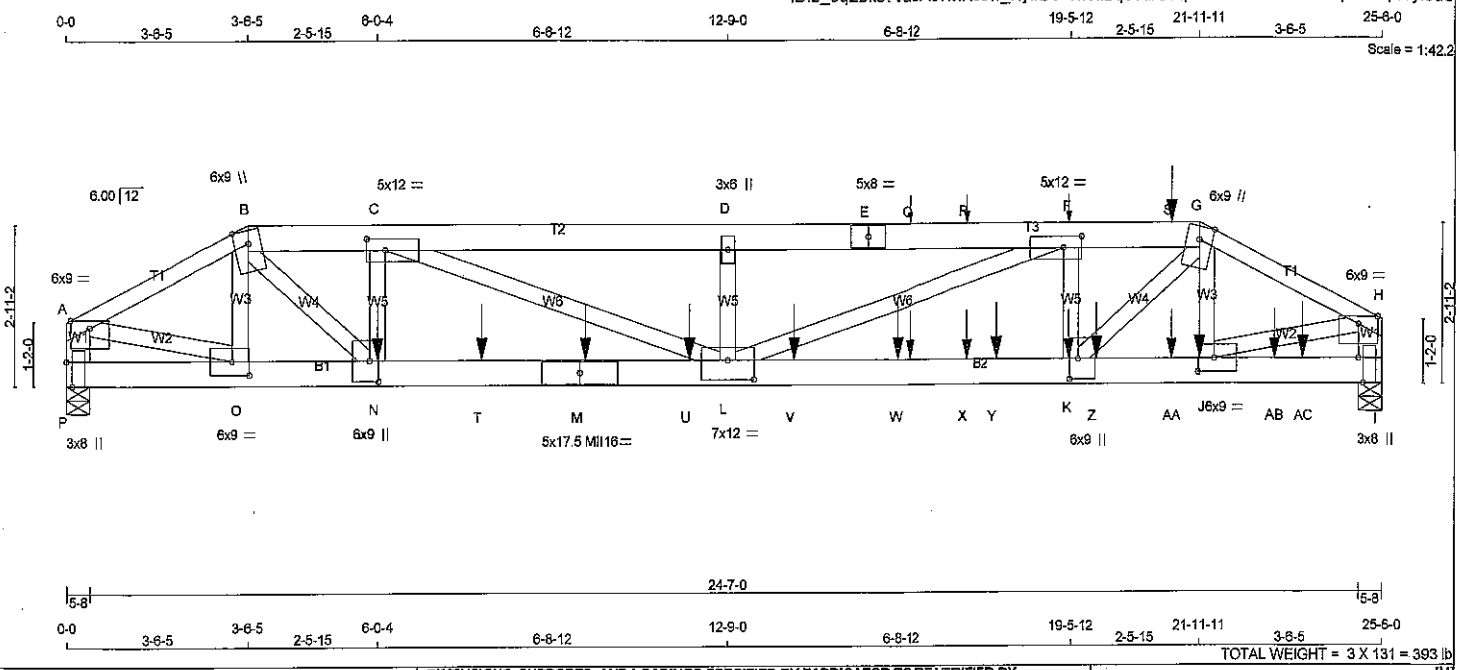

TOTAL WEIGHT = 3 X 131 = 393 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - E	2x6	DRY	No.2	SPF
E - G	2x6	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
P - A	2x6	DRY	No.2	SPF
I - H	2x6	DRY	No.2	SPF
P - M	2x6	DRY	2100F 1.8E	SPF
M - I	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS		SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- B	1	12	TOP
G- H	1	12	TOP
B- E	2	12	TOP
E- G	2	12	SIDE(0.2)
P- A	2	12	TOP
I- H	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
P- M	2	7	SIDE(612.6)
M- I	2	7	SIDE(1125.6)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	2	4	
C- N	2	4	SIDE(205.4)
J- G	1	6	
O- B	1	6	
N- B	1	6	
L- C	1	6	
D- L	1	6	
L- F	1	6	
K- G	1	6	
O- A	1	6	
J- H	1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-p	MT20	6.0	9.0	Edge	
B	TTWW+m	MT20	6.0	9.0	Edge	
C	TMWW-t	MT20	5.0	12.0	2.50	4.25
D	TMW+w	MT20	3.0	6.0		
E	TS-t	MT20	5.0	8.0		
F	TMWW-t	MT20	5.0	12.0	2.50	4.25
G	TTWW+m	MT20	6.0	9.0	Edge	
H	TMWW-p	MT20	6.0	9.0	Edge	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	8604	0	8604	0	0	5-8	5-8
I	10335	0	10335	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEA0	SOIL
P	6989	4002 / 0	1510 / 0	0 / 0	0 / 0	1477 / 0	0 / 0
I	8402	4800 / 0	1824 / 0	0 / 0	0 / 0	1779 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.42 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO							FR-TO		
A- B	-12898 / 0	-84.3	-84.3	0.28 (1)	3.27		O- B	-3727 / 0	0.15 (1)
B- C	-20981 / 0	-84.3	-84.3	0.27 (1)	3.21		B- N	0 / 13258	0.71 (1)
C- D	-28382 / 0	-84.3	-84.3	0.74 (1)	2.42		N- C	-3529 / 0	0.14 (1)
D- E	-28382 / 0	-84.3	-84.3	0.73 (1)	2.42		C- L	0 / 7887	0.42 (1)
E- Q	-28382 / 0	-84.3	-84.3	0.73 (1)	2.42		L- D	-373 / 56	0.01 (1)
Q- R	-28382 / 0	-84.3	-84.3	0.73 (1)	2.42		L- F	0 / 8068	0.43 (1)
R- F	-28382 / 0	-84.3	-84.3	0.73 (1)	2.42		K- F	-3616 / 0	0.14 (1)
F- S	-20811 / 0	-84.3	-84.3	0.28 (1)	3.22		K- G	0 / 11133	0.60 (1)
S- G	-20811 / 0	-84.3	-84.3	0.28 (1)	3.22		J- G	-1491 / 0	0.08 (1)
G- H	-14325 / 0	-84.3	-84.3	0.34 (1)	3.03		A- O	0 / 11696	0.83 (1)
P- A	-8355 / 0	0.0	0.0	0.18 (1)	6.27		J- H	0 / 13195	0.71 (1)
I- H	-9406 / 0	0.0	0.0	0.20 (1)	5.97				
P- O	0 / 0	-28.0	-28.0	0.06 (1)	10.00				
O- N	0 / 11617	-28.0	-28.0	0.42 (1)	10.00				
N- T	0 / 20978	-28.0	-28.0	0.64 (1)	10.00				
T- M	0 / 20978	-28.0	-28.0	0.64 (1)	10.00				
M- U	0 / 20978	-28.0	-28.0	0.64 (1)	10.00				
U- L	0 / 20978	-28.0	-28.0	0.64 (1)	10.00				
L- V	0 / 20809	-28.0	-28.0	0.62 (1)	10.00				
V- W	0 / 20809	-28.0	-28.0	0.62 (1)	10.00				
W- X	0 / 20809	-28.0	-28.0	0.62 (1)	10.00				
X- Y	0 / 20809	-28.0	-28.0	0.62 (1)	10.00				
Y- K	0 / 20809	-28.0	-28.0	0.62 (1)	10.00				
K- Z	0 / 12946	-28.0	-28.0	0.46 (1)	10.00				
Z-AA	0 / 12946	-28.0	-28.0	0.46 (1)	10.00				
AA- J	0 / 12946	-28.0	-28.0	0.46 (1)	10.00				
J-AB	0 / 0	-28.0	-28.0	0.17 (1)	10.00				
AB-AC	0 / 0	-28.0	-28.0	0.17 (1)	10.00				
AC-1	0 / 0	-28.0	-28.0	0.17 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
F	19-5-4	—	—	—	FRONT	VERT	TOTAL
J	21-11-12	-1438	-1438	—	BACK	VERT	TOTAL
K	19-5-4	-1	-1	—	FRONT	VERT	TOTAL
M	10-0-4	-1566	-1566	—	BACK	VERT	TOTAL
N	8-0-4	-2865	-2865	—	BACK	VERT	TOTAL
Q	16-3-4	—	—	—	FRONT	VERT	TOTAL
R	17-5-4	—	—	—	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM41835-17
STRUCTURAL COMPONENT ONLY

P6/6

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.85")
CALCULATED VERT. DEFL.(LL) = L/ 750 (0.41")
ALLOWABLE DEFL.(TL)= L/360 (0.85")
CALCULATED VERT. DEFL.(TL) = L/ 472 (0.65")

CSI: TC=0.74 (C-D:1) , BC=0.64 (L-N:1) , WB=0.71 (B-N:1) , SSI=0.56 (L-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	822	2264	1656
Mil16	473	276	2341	1245	4454	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (G) (INPUT = 0.90)
JSI METAL= 0.90 (M) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285414	T24	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
I	BMV1+p	MT20	3.0	8.0	5.50	1.00
J	BMWW-t	MT20	6.0	9.0	3.00	3.75
K	BMWW+t	MT20	6.0	9.0	4.50	2.00
L	BMWWW-t	MT20	7.0	12.0	4.25	6.00
M	BS-t	MI16	5.0	17.5		
N	BMWW+t	MT20	6.0	9.0	4.50	2.00
O	BMWW-t	MT20	6.0	9.0	3.00	3.75
P	BMV1+p	MT20	3.0	8.0	5.50	1.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) AT 16-3-4, AT 17-5-4, AND AT 19-5-4, AND 8.0 lbs FACTORED DOWN AT 21-5-4 ON TOP CHORD, AND 2865.3 lbs FACTORED DOWN AT 6-0-4, 1565.6 lbs FACTORED DOWN AT 8-0-4, 1565.6 lbs FACTORED DOWN AT 10-0-4, 1438.3 lbs FACTORED DOWN AT 12-0-4, 1438.3 lbs FACTORED DOWN AT 14-0-4, 1438.3 lbs FACTORED DOWN AT 16-0-4, 1.3 lbs FACTORED DOWN AT 16-3-4, 1.3 lbs FACTORED DOWN AT 17-5-4, 1438.3 lbs FACTORED DOWN AT 18-0-4, 1.3 lbs FACTORED DOWN AT 19-5-4, 1438.3 lbs FACTORED DOWN AT 19-11-12, 1.3 lbs FACTORED DOWN AT 21-5-4, 1438.3 lbs FACTORED DOWN AT 21-11-12, AND 1.3 lbs FACTORED DOWN AT 23-5-4, AND 1438.3 lbs FACTORED DOWN AT 23-11-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
S	21-5-4	-8	-8	---	FRONT	VERT	TOTAL
T	8-0-4	-1566	-1566	---	BACK	VERT	TOTAL
U	12-0-4	-1438	-1438	---	BACK	VERT	TOTAL
V	14-0-4	-1438	-1438	---	BACK	VERT	TOTAL
W	16-0-4	-1438	-1438	---	BACK	VERT	TOTAL
W	16-3-4	-1	-1	---	FRONT	VERT	TOTAL
X	17-5-4	-1	-1	---	FRONT	VERT	TOTAL
Y	18-0-4	-1438	-1438	---	BACK	VERT	TOTAL
Z	19-11-12	-1438	-1438	---	BACK	VERT	TOTAL
AA	21-5-4	-1	-1	---	FRONT	VERT	TOTAL
AB	23-5-4	-1	-1	---	FRONT	VERT	TOTAL
AC	23-11-12	-1438	-1438	---	BACK	VERT	TOTAL

DWG NO. TAM 41835-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285415	T25	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Dec 12 2017 MiTek Industries, Inc. Tue Feb 6 11:09:13 2018 Page 1

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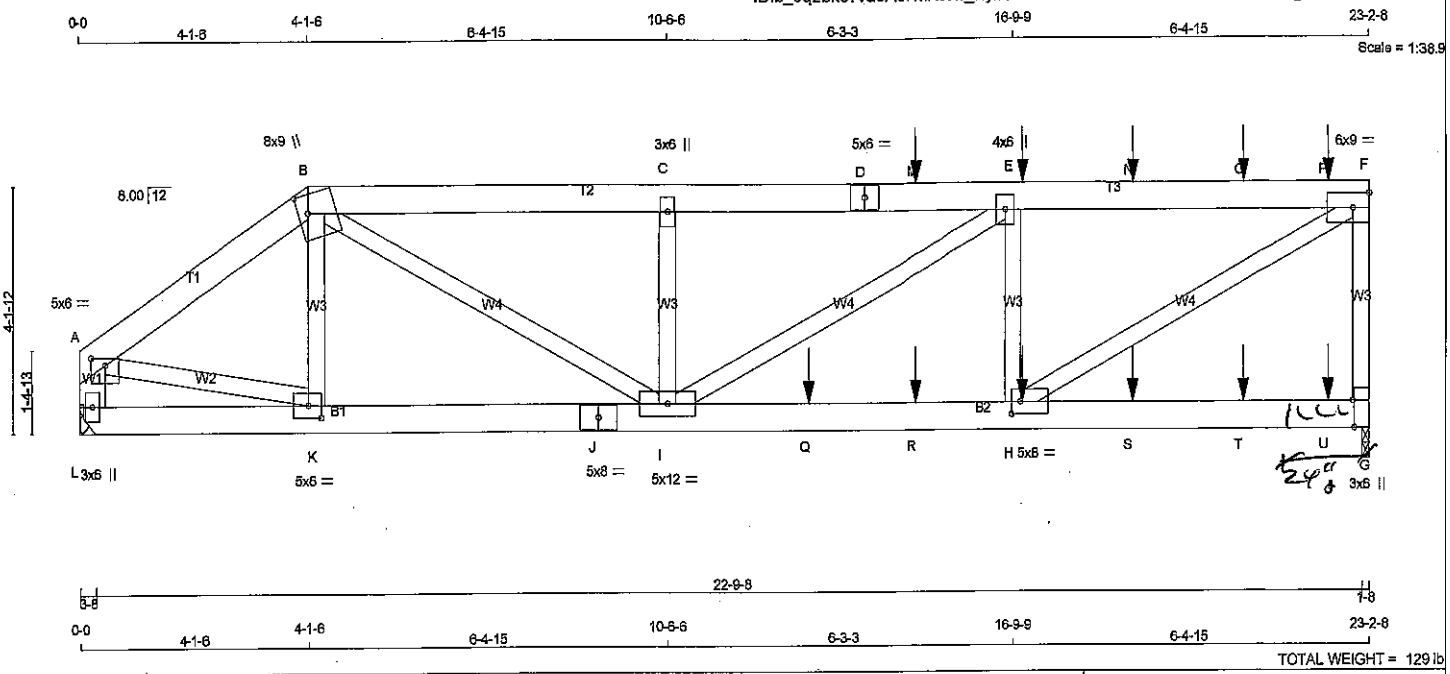

TOTAL WEIGHT = 129 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x6	DRY	No.2	SPF
B - D	2x6	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - A	2x6	DRY	No.2	SPF
L - J	2x6	DRY	2100F 1.8E	SPF
J - G	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	6.0	1.50	3.00
B	TTWW+m	MT20	8.0	9.0	3.75	2.00
C	TMW+w	MT20	3.0	6.0		
D	TS-t	MT20	5.0	6.0		
E	TMWW+t	MT20	4.0	6.0		
F	TMVW-t	MT20	6.0	9.0		Edge
G	BMV1+p	MT20	3.0	8.0	Edge	0.25
H	BMWW-t	MT20	5.0	8.0	2.50	2.00
I	BMWWW-t	MT20	5.0	12.0		
J	BS-t	MT20	5.0	8.0		
K	BMWW-t	MT20	5.0	6.0	2.50	2.75
L	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 104.0 lbs FACTORED DOWN AT 14-11-12, 104.0 lbs FACTORED DOWN AT 16-11-12, 104.0 lbs FACTORED DOWN AT 18-11-12, AND 104.0 lbs FACTORED DOWN AT 20-11-12, AND 115.4 lbs FACTORED DOWN AT 22-5-12 ON TOP CHORD, AND 1284.8 lbs FACTORED DOWN AT 13-0-8, 71.5 lbs FACTORED DOWN AT 14-11-12, 71.5 lbs FACTORED DOWN AT 16-11-12, 71.5 lbs FACTORED DOWN AT 18-11-12, AND 71.5 lbs FACTORED DOWN AT 20-11-12, AND 78.1 lbs FACTORED DOWN AT 22-5-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

SCAB JOINT # G & ~~F~~ WITH 2 X 6 #2 SPF 24" LONG 1 SIDE(S) USING 2 ROW(S) 3½" ARDOX SPIRAL/~~COMMON WIRE~~ NAILS AT 3" O/C STAGGERED (15 NAILS/SCAB). REQUIRED FOR 1½" BRG PROVISION.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
G	2628	0	2628	0	0	1-8	6-8
L	2002	0	2002	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	IN-SX

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	2150	1206 / 0	482 / 0	0 / 0	0 / 0	462 / 0	0 / 0
L	1632	925 / 0	359 / 0	0 / 0	0 / 0	348 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.71 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	-2388 / 0	-84.3	-84.3	0.17 (1)	5.15	K-B	-406 / 15	0.07 (1)
B-C	-4270 / 0	-84.3	-84.3	0.39 (1)	3.85	B-I	0 / 2654	0.47 (1)
C-D	-4270 / 0	-84.3	-84.3	0.49 (1)	3.71	I-C	-553 / 0	0.10 (1)
D-M	-4270 / 0	-84.3	-84.3	0.49 (1)	3.71	I-E	0 / 630	0.11 (1)
M-E	-4270 / 0	-84.3	-84.3	0.49 (1)	3.71	H-E	-1253 / 0	0.22 (1)
E-N	-3728 / 0	-84.3	-84.3	0.51 (1)	3.89	H-F	0 / 4300	0.76 (1)
N-O	-3728 / 0	-84.3	-84.3	0.51 (1)	3.89	A-K	0 / 2025	0.36 (1)
O-P	-3728 / 0	-84.3	-84.3	0.51 (1)	3.89			
P-F	-3728 / 0	-84.3	-84.3	0.51 (1)	3.89			
G-F	-2549 / 0	0.0	0.0	0.60 (1)	5.31			
L-A	-1923 / 0	0.0	0.0	0.14 (1)	7.22			
L-K	0 / 0	-28.0	-28.0	0.05 (1)	10.00			
K-J	0 / 1966	-28.0	-28.0	0.36 (1)	10.00			
J-I	0 / 1966	-28.0	-28.0	0.36 (1)	10.00			
I-Q	0 / 3726	-28.0	-28.0	0.78 (1)	10.00			
Q-R	0 / 3726	-28.0	-28.0	0.78 (1)	10.00			
R-H	0 / 3726	-28.0	-28.0	0.78 (1)	10.00			
H-S	0 / 0	-28.0	-28.0	0.24 (2)	10.00			
S-T	0 / 0	-28.0	-28.0	0.24 (2)	10.00			
T-U	0 / 0	-28.0	-28.0	0.24 (2)	10.00			
U-G	0 / 0	-28.0	-28.0	0.24 (2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
E	16-11-12	-104	-104	---	FRONT	VERT	TOTAL
H	16-11-12	-41	-71	---	FRONT	VERT	TOTAL
M	14-11-12	-104	-104	---	FRONT	VERT	TOTAL
N	18-11-12	-104	-104	---	FRONT	VERT	TOTAL
O	20-11-12	-104	-104	---	FRONT	VERT	TOTAL
P	22-5-12	-115	-115	---	FRONT	VERT	TOTAL
Q	13-0-8	-1285	-1285	---	FRONT	VERT	TOTAL
R	14-11-12	-41	-71	---	FRONT	VERT	TOTAL
S	18-11-12	-41	-71	---	FRONT	VERT	TOTAL
T	20-11-12	-41	-71	---	FRONT	VERT	TOTAL
U	22-5-12	-45	-78	---	FRONT	VERT	TOTAL

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.77")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.23")
ALLOWABLE DEFL.(TL)= L/360 (0.77")
CALCULATED VERT. DEFL.(TL) = L/ 761 (0.37")

CSI: TC=0.60/1.00 (F-G:1) , BC=0.78/1.00 (H-I:1) , WB=0.76/1.00 (F-H:1) , SSI=0.58/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.70 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM B100-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285415	T26	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Wed May 16 08:38:34 2018 Page 1

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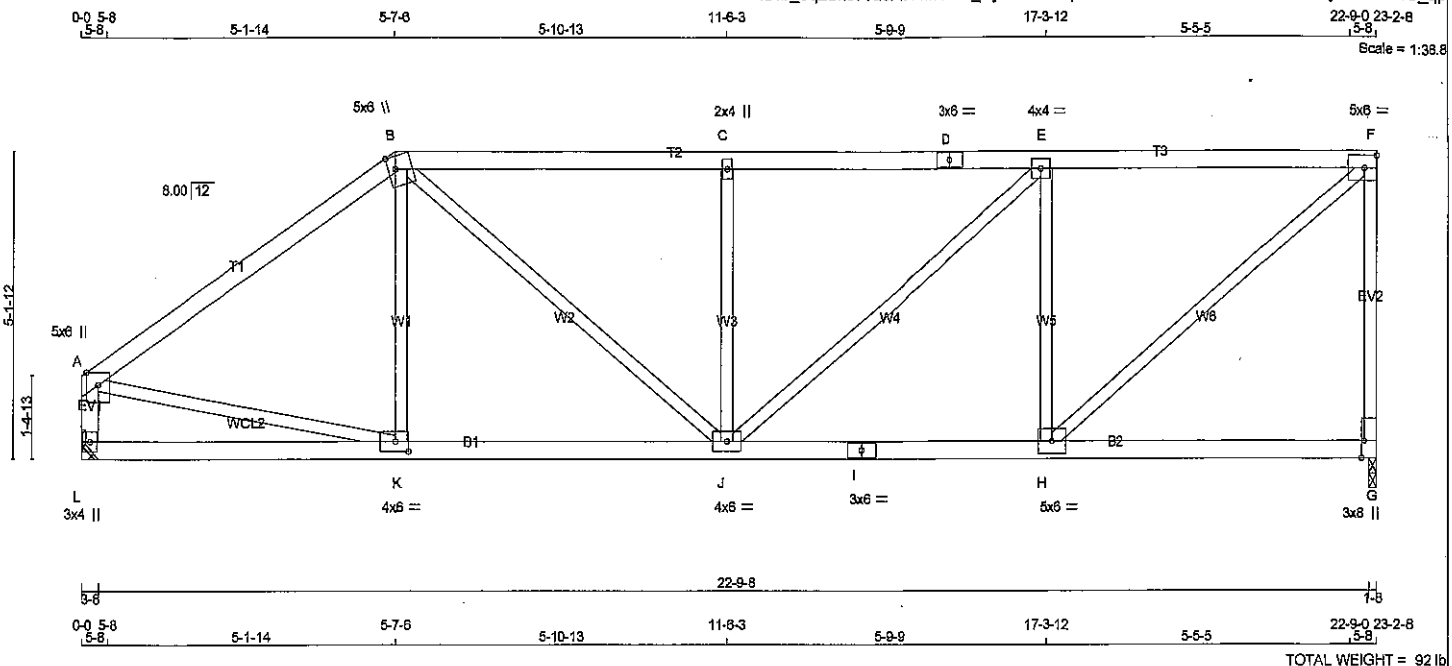

TOTAL WEIGHT = 92 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x3	DRY	No.2	SPF
L - A	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	Edge	
B	TTWW+m	MT20	5.0	6.0	2.50	1.50
C	TMW+w	MT20	2.0	4.0		
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMVW-t	MT20	5.0	6.0		Edge
G	BMV1+t	MT20	3.0	8.0	Edge	0.50
H	BMWW-t	MT20	5.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BMWW-t	MT20	4.0	8.0	2.00	2.75
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT	REQRD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	BRG
G	1303	0	1303	0	0	1-8	1-8
L	1303	0	1303	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1070	594 / 0	244 / 0	0 / 0	0 / 0	232 / 0	0 / 0
L	1070	594 / 0	244 / 0	0 / 0	0 / 0	232 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.64 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO									
A- B	-1406 / 0	-84.3	-84.3	0.56 (1)	4.80	K- B	-36 / 206	0.05 (3)	
B- C	-1625 / 0	-84.3	-84.3	0.47 (1)	4.66	B- J	0 / 592	0.13 (1)	
C- D	-1625 / 0	-84.3	-84.3	0.47 (1)	4.64	J- C	-535 / 0	0.21 (1)	
D- E	-1625 / 0	-84.3	-84.3	0.47 (1)	4.64	J- E	0 / 480	0.11 (1)	
E- F	-1257 / 0	-84.3	-84.3	0.44 (1)	5.16	H- E	-849 / 0	0.33 (1)	
G- F	-1236 / 0	0.0	0.0	0.78 (1)	6.33	H- F	0 / 1628	0.37 (1)	
L- A	-1239 / 0	0.0	0.0	0.13 (1)	7.22	A- K	0 / 1189	0.27 (1)	
L- K	0 / 0	-28.0	-28.0	0.23 (3)	10.00				
K- J	0 / 1167	-28.0	-28.0	0.37 (2)	10.00				
J- I	0 / 1257	-28.0	-28.0	0.38 (2)	10.00				
I- H	0 / 1257	-28.0	-28.0	0.36 (2)	10.00				
H- G	0 / 0	-28.0	-28.0	0.24 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.77")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.77")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.78/1.00 (F-G:1) , BC=0.38/1.00 (H-J:2) , WB=0.37/1.00 (F-H:1) , SSI=0.23/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (A) (INPUT = 0.90)
JSI METAL= 0.41 (I) (INPUT = 1.00)

DWG NO. TAM 26867-R8
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285414	T27	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Wed Aug 23 09:35:30 2017 Page 1

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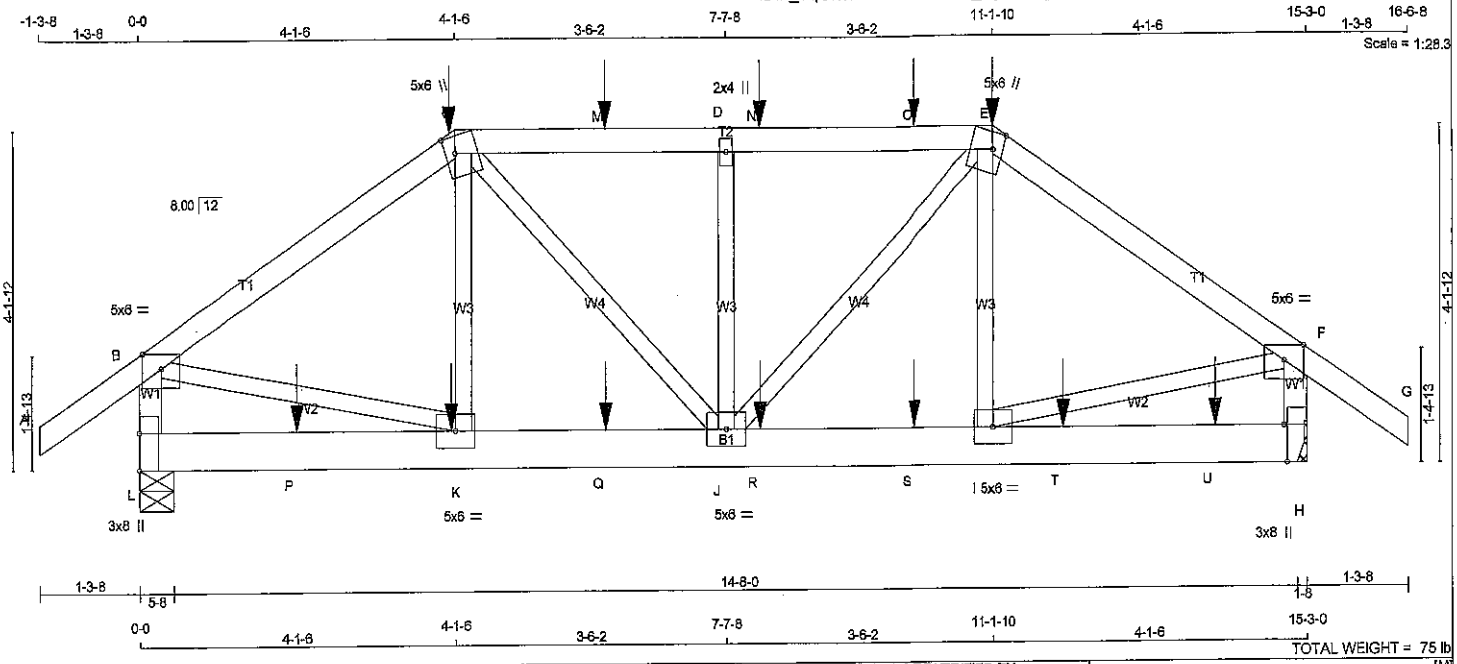

TOTAL WEIGHT = 75 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.50	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.50	1.50
F	TMVW-p	MT20	5.0	6.0	Edge	
H	BMV1+t	MT20	3.0	8.0	Edge	0.50
I	BMWW-t	MT20	5.0	6.0		
J	BMWWW-t	MT20	5.0	6.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMV1+t	MT20	3.0	8.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 356.6 lbs FACTORED DOWN AT 4-1-6, 249.0 lbs FACTORED DOWN AT 11-1-10, 104.0 lbs FACTORED DOWN AT 6-0-12, AND 104.0 lbs FACTORED DOWN AT 8-0-12, AND 104.0 lbs FACTORED DOWN AT 10-0-12 ON TOP CHORD, AND 71.5 lbs FACTORED DOWN AT 2-0-12, 71.5 lbs FACTORED DOWN AT 4-0-12, 71.5 lbs FACTORED DOWN AT 6-0-12, 71.5 lbs FACTORED DOWN AT 8-0-12, 71.5 lbs FACTORED DOWN AT 10-0-12, AND 71.5 lbs FACTORED DOWN AT 12-0-12, AND 71.5 lbs FACTORED DOWN AT 14-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
L	1581	0	1581	0	0	IN-SX	IN-SX
H	1566	0	1566	0	0	5-8	5-8
						HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1272	747 / 0	251 / 0	0 / 0	0 / 0	264 / 0	0 / 0
H	1272	729 / 0	274 / 0	0 / 0	0 / 0	269 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 32	-84.3	-84.3	0.12 (1)	10.00	K- C	-103 / 217	0.05 (3)
B- C	-1622 / 0	-84.3	-84.3	0.33 (1)	4.80	C- J	0 / 448	0.11 (1)
C- M	-1650 / 0	-84.3	-84.3	0.33 (1)	4.75	J- D	-568 / 0	0.15 (1)
M- D	-1650 / 0	-84.3	-84.3	0.33 (1)	4.75	J- E	0 / 494	0.12 (1)
D- N	-1650 / 0	-84.3	-84.3	0.33 (1)	4.75	I- E	-89 / 225	0.06 (3)
N- O	-1650 / 0	-84.3	-84.3	0.33 (1)	4.75	B- K	0 / 384	0.34 (1)
O- E	-1650 / 0	-84.3	-84.3	0.33 (1)	4.75	I- F	0 / 351	0.33 (1)
E- F	-1583 / 0	-84.3	-84.3	0.33 (1)	4.85			
F- G	0 / 32	-84.3	-84.3	0.12 (1)	10.00			
L- B	-1510 / 0	0.0	0.0	0.17 (1)	6.64			
H- F	-1481 / 0	0.0	0.0	0.17 (1)	6.70			
L- P	0 / 0	-28.0	-28.0	0.10 (2)	10.00			
P- K	0 / 0	-28.0	-28.0	0.10 (2)	10.00			
K- Q	0 / 1344	-28.0	-28.0	0.23 (1)	10.00			
Q- J	0 / 1344	-28.0	-28.0	0.23 (1)	10.00			
J- R	0 / 1313	-28.0	-28.0	0.21 (1)	10.00			
R- S	0 / 1313	-28.0	-28.0	0.21 (1)	10.00			
S- I	0 / 1313	-28.0	-28.0	0.21 (1)	10.00			
I- T	0 / 0	-28.0	-28.0	0.10 (2)	10.00			
T- U	0 / 0	-28.0	-28.0	0.10 (2)	10.00			
U- H	0 / 0	-28.0	-28.0	0.10 (2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	4-1-6	-23	-25	—	FRONT	VERT	DEAD
C	4-1-6	-108	-108	—	FRONT	VERT	TOTAL
C	4-1-6	-227	-227	—	FRONT	VERT	SNOW
E	11-1-10	-23	-25	—	FRONT	VERT	DEAD
E	11-1-10	-227	-227	—	FRONT	VERT	SNOW
K	4-0-12	-41	-71	—	FRONT	VERT	TOTAL
M	6-0-12	-104	-104	—	FRONT	VERT	TOTAL
N	8-0-12	-104	-104	—	FRONT	VERT	TOTAL
O	10-0-12	-104	-104	—	FRONT	VERT	TOTAL
P	2-0-12	-41	-71	—	FRONT	VERT	TOTAL
Q	6-0-12	-41	-71	—	FRONT	VERT	TOTAL
R	8-0-12	-41	-71	—	FRONT	VERT	TOTAL
S	10-0-12	-41	-71	—	FRONT	VERT	TOTAL
T	12-0-12	-41	-71	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.51")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.51")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.33 (B-C:1) , BC=0.23 (J-K:1) , WB=0.34 (B-K:1) , SSI=0.28 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (B) (INPUT = 0.90)
JSI METAL= 0.34 (I) (INPUT = 1.00)

PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 4183817

STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285414	T27	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
U	14-0-12	-41	-71	—	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. T27 4/838-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285414	T28	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Wed Aug 23 09:35:30 2017 Page 1

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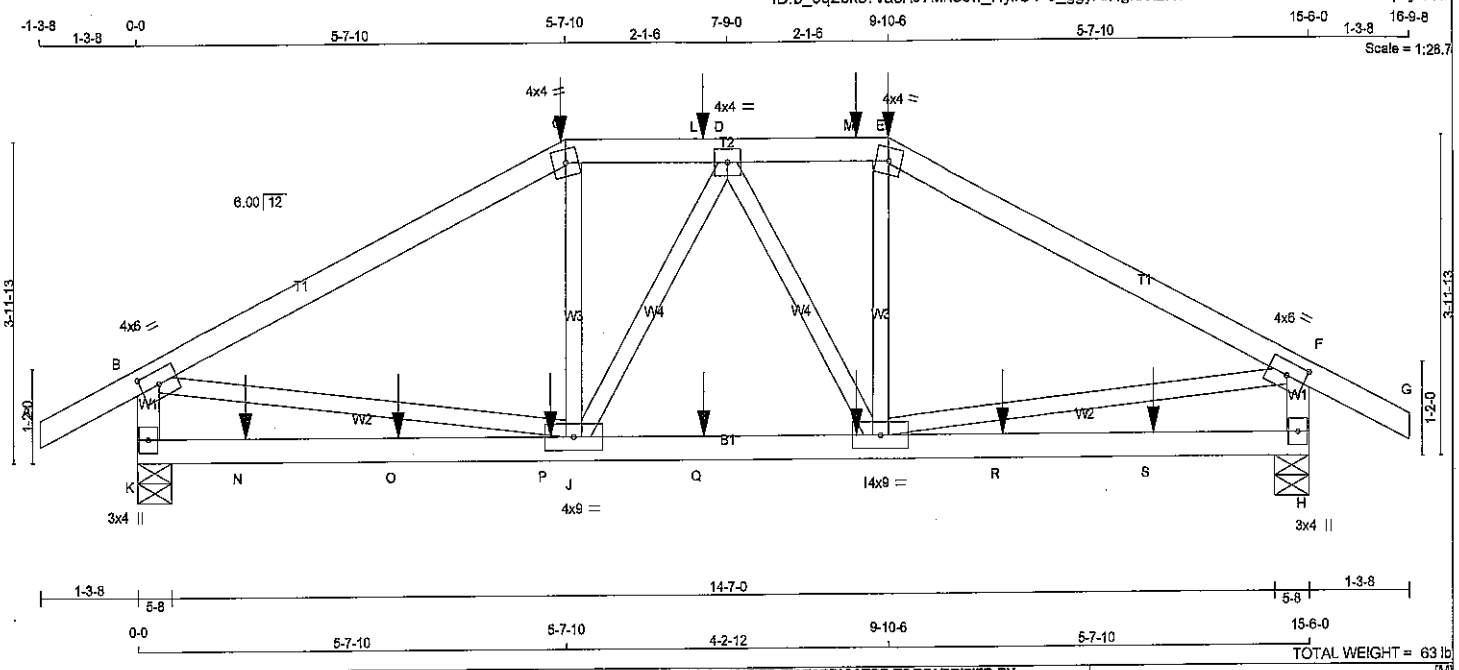

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	2.00	3.00
C	TTW-m	MT20	4.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TTW-m	MT20	4.0	4.0		
F	TMVW-t	MT20	4.0	6.0	2.00	3.00
H	BMV1+p	MT20	3.0	4.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BMWWW-t	MT20	4.0	9.0		
K	BMV1+p	MT20	3.0	4.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 241.6 lbs FACTORED DOWN AT 5-7-10, 200.3 lbs FACTORED DOWN AT 9-10-6, AND 38.3 lbs FACTORED DOWN AT 7-5-4, AND 62.4 lbs FACTORED DOWN AT 9-5-4 ON TOP CHORD, AND 33.0 lbs FACTORED DOWN AT 1-5-4, 33.0 lbs FACTORED DOWN AT 3-5-4, 33.0 lbs FACTORED DOWN AT 5-5-4, 33.0 lbs FACTORED DOWN AT 7-5-4, 33.0 lbs FACTORED DOWN AT 9-5-4, AND 33.0 lbs FACTORED DOWN AT 11-5-4, AND 33.0 lbs FACTORED DOWN AT 13-5-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	1324	0	1324	0	0	5-8	5-8
H	1320	0	1320	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	1062	629 / 0	214 / 0	0 / 0	0 / 0	219 / 0	0 / 0
H	1056	630 / 0	211 / 0	0 / 0	0 / 0	216 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.49 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 26	-84.3	-84.3	0.12 (1)	10.00	J- C	0 / 257	0.06 (3)
B- C	-1520 / 0	-84.3	-84.3	0.61 (1)	4.49	J- D	-144 / 0	0.04 (1)
C- L	-1364 / 0	-84.3	-84.3	0.09 (1)	5.45	D- I	-142 / 0	0.04 (1)
L- D	-1364 / 0	-84.3	-84.3	0.09 (1)	5.45	I- E	0 / 259	0.06 (3)
D- M	-1366 / 0	-84.3	-84.3	0.09 (1)	5.44	B- J	0 / 1374	0.34 (1)
M- E	-1366 / 0	-84.3	-84.3	0.09 (1)	5.44	I- F	0 / 1375	0.34 (1)
E- F	-1522 / 0	-84.3	-84.3	0.61 (1)	4.49			
F- G	0 / 26	-84.3	-84.3	0.12 (1)	10.00			
K- B	-1234 / 0	0.0	0.0	0.14 (1)	7.19			
H- F	-1235 / 0	0.0	0.0	0.14 (1)	7.19			
K- N	0 / 0	-28.0	-28.0	0.29 (2)	10.00			
N- O	0 / 0	-28.0	-28.0	0.29 (2)	10.00			
O- P	0 / 0	-28.0	-28.0	0.29 (2)	10.00			
P- J	0 / 0	-28.0	-28.0	0.29 (2)	10.00			
J- Q	0 / 1432	-28.0	-28.0	0.43 (2)	10.00			
Q- I	0 / 1432	-28.0	-28.0	0.43 (2)	10.00			
I- R	0 / 0	-28.0	-28.0	0.29 (2)	10.00			
R- S	0 / 0	-28.0	-28.0	0.29 (2)	10.00			
S- H	0 / 0	-28.0	-28.0	0.29 (2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
C	5-7-10	-19	-21	—	FRONT	VERT	DEAD
C	5-7-10	-41	-41	—	FRONT	VERT	TOTAL
C	5-7-10	-182	-182	—	FRONT	VERT	SNOW
E	9-10-6	-19	-21	—	FRONT	VERT	DEAD
E	9-10-6	-182	-182	—	FRONT	VERT	SNOW
I	9-5-4	-19	-33	—	FRONT	VERT	TOTAL
L	7-5-4	-38	-38	—	FRONT	VERT	TOTAL
M	9-5-4	-62	-62	—	FRONT	VERT	TOTAL
N	1-5-4	-19	-33	—	FRONT	VERT	TOTAL
O	3-5-4	-19	-33	—	FRONT	VERT	TOTAL
P	5-5-4	-19	-33	—	FRONT	VERT	TOTAL
Q	7-5-4	-19	-33	—	FRONT	VERT	TOTAL
R	11-5-4	-19	-33	—	FRONT	VERT	TOTAL
S	13-5-4	-19	-33	—	FRONT	VERT	TOTAL

[M]

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.52")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.52")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.61 (E-F:1) , BC=0.43 (I-J:2) , WB=0.34 (F-I:1) , SSI=0.20 (J-K:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	822	2284	1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (I) (INPUT = 0.90)
JSI METAL= 0.42 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM-171034-17
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285414	T29	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Wed Aug 23 09:35:31 2017 Page 1

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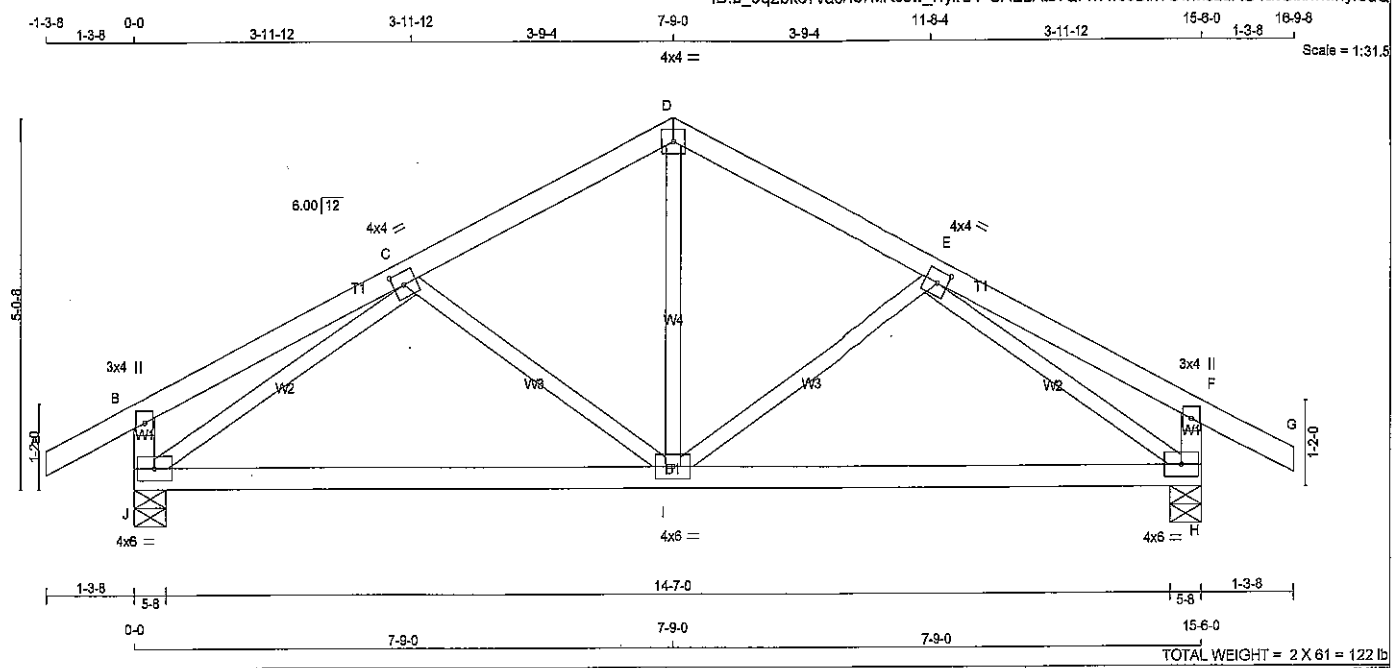

TOTAL WEIGHT = 2 X 61 = 122 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTW-p	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.75
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	984	0	984	0	0	5-8	5-8
H	984	0	984	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	792	466 / 0	163 / 0	0 / 0	0 / 0	163 / 0	0 / 0
H	792	466 / 0	163 / 0	0 / 0	0 / 0	163 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 26	-84.3	-84.3	0.11 (1)	10.00	I- D	0 / 501	0.11 (1)
B- C	0 / 16	-84.3	-84.3	0.19 (1)	10.00	I- E	-198 / 55	0.08 (1)
C- D	-859 / 0	-84.3	-84.3	0.16 (1)	6.25	C- I	-198 / 55	0.08 (1)
D- E	-859 / 0	-84.3	-84.3	0.16 (1)	6.25	J- C	-1127 / 0	0.42 (1)
E- F	0 / 16	-84.3	-84.3	0.19 (1)	10.00	E- H	-1127 / 0	0.42 (1)
F- G	0 / 26	-84.3	-84.3	0.11 (1)	10.00			
J- B	-242 / 0	0.0	0.0	0.02 (1)	7.81			
H- F	-242 / 0	0.0	0.0	0.02 (1)	7.81			
J- I	0 / 915	-28.0	-28.0	0.59 (2)	10.00			
I- H	0 / 915	-28.0	-28.0	0.59 (2)	10.00			

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. T2M41040-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.52")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.52")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.19 (B-C:1) , BC=0.59 (I-J:2) , WB=0.42 (C-J:1) , SSI=0.18 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (C) (INPUT = 0.90)
JSI METAL= 0.38 (C) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285414	T30A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Wed Aug 23 09:35:31 2017 Page 1

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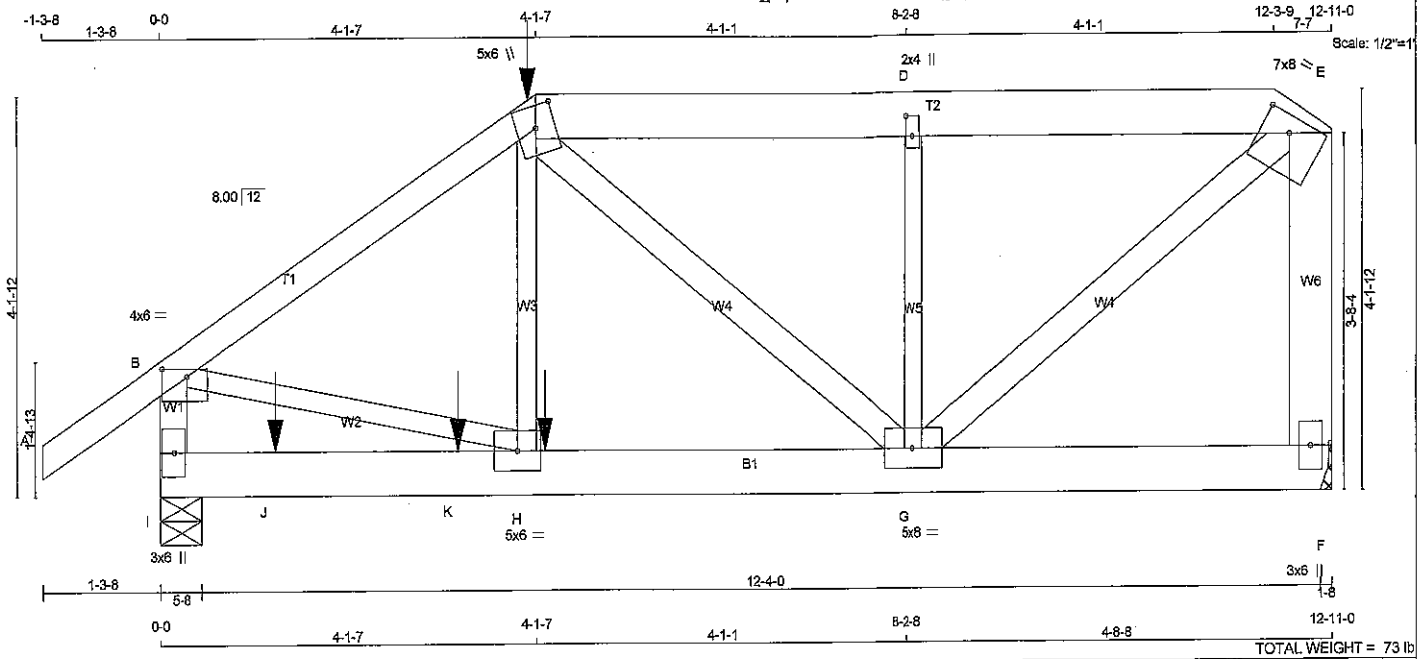

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
F - E	2x6	DRY	No.2	SPF
I - F	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	6.0	1.00	3.25
C	TTWW+m	MT20	5.0	6.0	2.75	2.50
D	TMW+w	MT20	2.0	4.0	2.50	1.00
E	TMVWm	MT20	7.0	8.0	2.00	3.50
F	BMV1+p	MT20	3.0	6.0		
G	BMWWW-t	MT20	5.0	8.0		
H	BMWW-t	MT20	5.0	6.0		
I	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 249.0 lbs FACTORED DOWN AT 4-1-6 ON TOP CHORD, AND 71.5 lbs FACTORED DOWN AT 1-3-4, AND 71.5 lbs FACTORED DOWN AT 3-3-4, AND 967.9 lbs FACTORED DOWN AT 4-2-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	1745	0	1745	0	0	5-8	5-8
F	1119	0	1119	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1388	843 / 0	268 / 0	0 / 0	0 / 0	278 / 0	0 / 0
F	899	531 / 0	183 / 0	0 / 0	0 / 0	185 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.60 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 32	-84.3	-84.3	0.12 (1)	10.00	H-C	0 / 738	0.18 (1)
B-C	-1792 / 0	-84.3	-84.3	0.34 (1)	4.60	C-G	-456 / 0	0.17 (1)
C-D	-1134 / 0	-84.3	-84.3	0.13 (1)	6.25	G-D	-451 / 0	0.11 (1)
D-E	-1134 / 0	-84.3	-84.3	0.13 (1)	6.25	B-H	0 / 1541	0.38 (1)
I-B	-1648 / 0	0.0	0.0	0.18 (1)	6.41	G-E	0 / 1450	0.26 (1)
F-E	-1064 / 0	0.0	0.0	0.16 (1)	7.81			
I-J	0 / 0	-28.0	-28.0	0.11 (2)	10.00			
J-K	0 / 0	-28.0	-28.0	0.11 (2)	10.00			
K-H	0 / 0	-28.0	-28.0	0.11 (2)	10.00			
H-G	0 / 1475	-28.0	-28.0	0.23 (1)	10.00			
G-F	0 / 0	-28.0	-28.0	0.07 (3)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	4-1-6	-23	-25	---	FRONT	VERT	DEAD
C	4-1-6	-227	-227	---	FRONT	VERT	SNOW
H	4-2-8	-968	-968	---	FRONT	VERT	TOTAL
J	1-3-4	-41	-71	---	FRONT	VERT	TOTAL
K	3-3-4	-41	-71	---	FRONT	VERT	TOTAL

DWG NO. TAM4/6911-17
STRUCTURAL COMPONENT ONLY

[M]

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.34 (B-C:1) , BC=0.23 (G-H:1) , WB=0.38 (B-H:1) , SSI=0.15 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (B) (INPUT = 0.90)
JSI METAL= 0.40 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285414	T31A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Wed Aug 23 09:35:31 2017 Page 1
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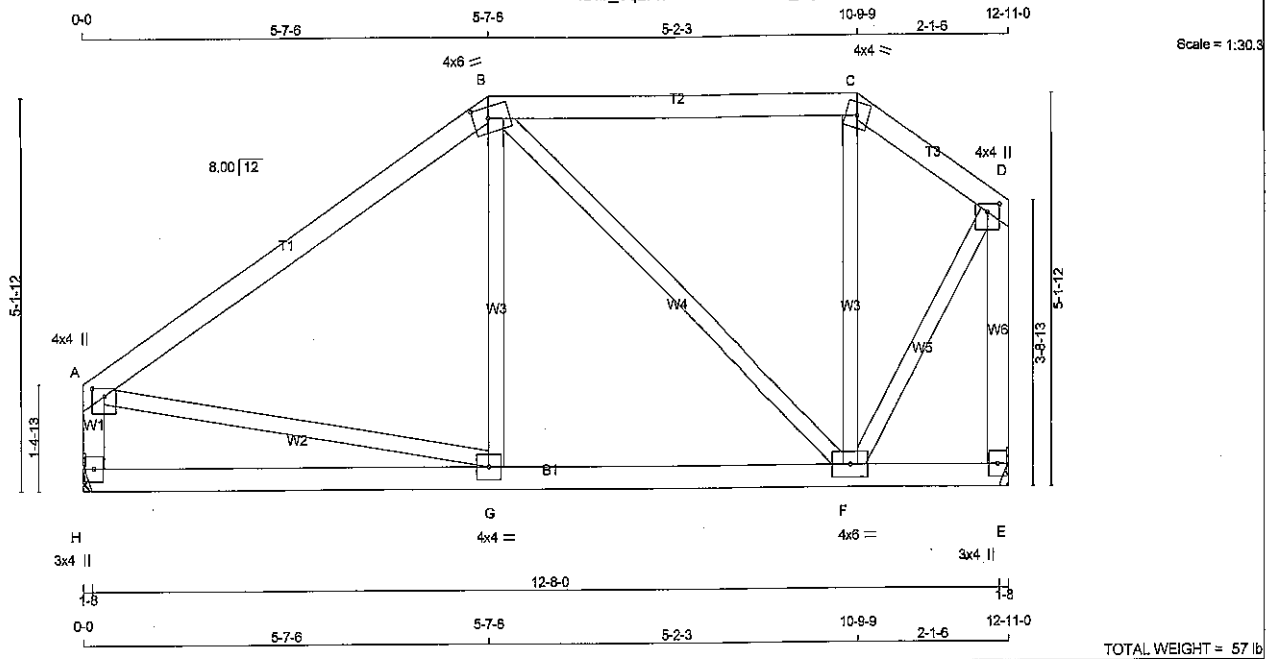


TOTAL WEIGHT = 57 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00
B	TTWW-m	MT20	4.0	6.0	1.75	2.50
C	TTW-m	MT20	4.0	4.0		
D	TMVW+p	MT20	4.0	4.0	1.25	2.00
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	6.0		
G	BMWW-t	MT20	4.0	4.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	725	0	725	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
E	725	0	725	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

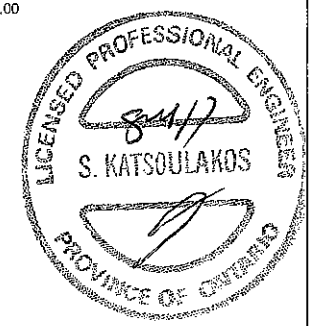
JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	595	330 / 0	136 / 0	0 / 0	0 / 0	129 / 0	0 / 0
E	595	330 / 0	136 / 0	0 / 0	0 / 0	129 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-594 / 0	-84.3	-84.3	0.35 (1)	6.25	G- B	0 / 260	0.06 (3)
B- C	-277 / 0	-84.3	-84.3	0.29 (1)	6.25	B- F	-301 / 0	0.27 (1)
C- D	-337 / 0	-84.3	-84.3	0.05 (1)	6.25	F- C	-119 / 73	0.05 (1)
H- A	-660 / 0	0.0	0.0	0.07 (1)	7.81	A- G	0 / 502	0.11 (1)
E- D	-707 / 0	0.0	0.0	0.16 (1)	7.81	F- D	0 / 513	0.12 (1)
H- G	0 / 0	-28.0	-28.0	0.22 (3)	10.00			
G- F	0 / 495	-28.0	-28.0	0.28 (2)	10.00			
F- E	0 / 0	-28.0	-28.0	0.06 (3)	10.00			



DWG NO. T31A 4/1642-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.35 (A-B:1) , BC=0.28 (F-G:2) , WB=0.27 (B-F:1) , SSI=0.17 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	822	2284	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.65 (F) (INPUT = 0.90)
JSI METAL= 0.19 (G) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285414	T32	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Wed Aug 23 09:35:32 2017 Page 1

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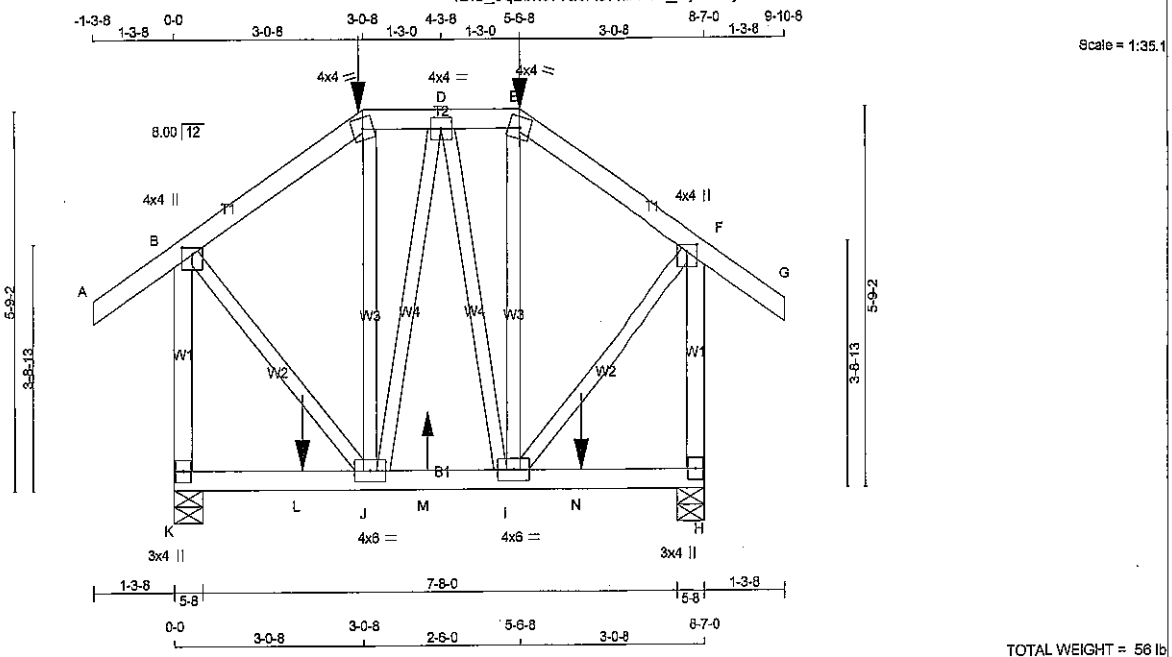

TOTAL WEIGHT = 56 lb

[M

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTW-m	MT20	4.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TTW-m	MT20	4.0	4.0		
F	TMVW+p	MT20	4.0	4.0	1.25	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 78.0 lbs FACTORED DOWN AT 3-0-8, AND 78.0 lbs FACTORED DOWN AT 5-6-8 ON TOP CHORD, AND 142.7 lbs FACTORED DOWN AT 2-0-12, AND 0.9 lbs FACTORED UP AT 4-0-12, AND 142.7 lbs FACTORED DOWN AT 6-6-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
K	818	0	818	0	0	5-8	5-8
H	818	0	818	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	832	414 / 0	101 / 0	0 / 0	0 / 0	116 / 0	0 / 0
H	632	414 / 0	101 / 0	0 / 0	0 / 0	116 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 32	-84.3	-84.3	0.12 (1)	10.00		J- C	-64 / 49	0.03 (1)
B- C	-342 / 0	-84.3	-84.3	0.15 (1)	6.25		J- D	-59 / 0	0.03 (1)
C- D	-283 / 0	-84.3	-84.3	0.02 (1)	6.25		D- I	-66 / 0	0.04 (1)
D- E	-283 / 0	-84.3	-84.3	0.02 (1)	6.25		I- E	-55 / 49	0.03 (1)
E- F	-342 / 0	-84.3	-84.3	0.15 (1)	6.25		B- J	0 / 419	0.10 (1)
F- G	0 / 32	-84.3	-84.3	0.12 (1)	10.00		I- F	0 / 419	0.10 (1)
K- B	-743 / 0	0.0	0.0	0.18 (1)	7.81				
H- F	-743 / 0	0.0	0.0	0.18 (1)	7.81				
K- L	0 / 0	-28.0	-28.0	0.14 (1)	10.00				
L- J	0 / 0	-28.0	-28.0	0.14 (1)	10.00				
J- M	0 / 296	-28.0	-28.0	0.14 (1)	10.00				
M- I	0 / 296	-28.0	-28.0	0.14 (1)	10.00				
I- N	0 / 0	-28.0	-28.0	0.14 (1)	10.00				
N- H	0 / 0	-28.0	-28.0	0.14 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE
C	3-0-8	-8	-8	---	FRONT	VERT	DEAD
C	3-0-8	-71	-71	---	FRONT	VERT	SNOW
E	5-6-6	-8	-8	---	FRONT	VERT	DEAD
E	5-6-8	-71	-71	---	FRONT	VERT	SNOW
L	2-0-12	-143	-143	---	FRONT	VERT	TOTAL
M	4-0-12	1	---	1	FRONT	VERT	TOTAL
N	6-6-4	-143	-143	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.8	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.29")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.29")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.18 (F-H:1) , BC=0.14 (J-K:1) , WB=0.10 (F-I:1) , SSI=0.13 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.53 (J) (INPUT = 0.90)
JSI METAL= 0.11 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 4/04/3-17
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285414	T33	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Wed Aug 23 09:35:32 2017 Page 1

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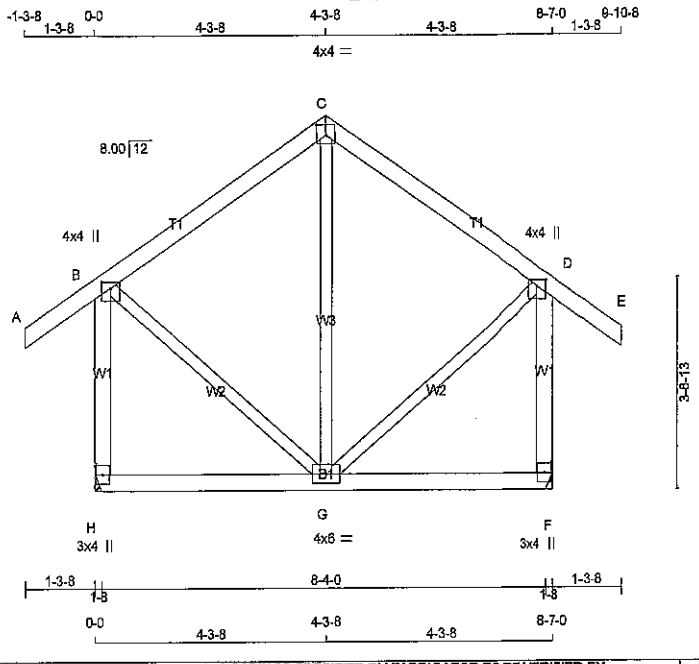


TOTAL WEIGHT = 46 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMVW+p	MT20	4.0	4.0	1.25	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	597	0	597	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
F	597	0	597	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	474	290 / 0	90 / 0	0 / 0	0 / 0	94 / 0	0 / 0
F	474	290 / 0	90 / 0	0 / 0	0 / 0	94 / 0	0 / 0

BRACING

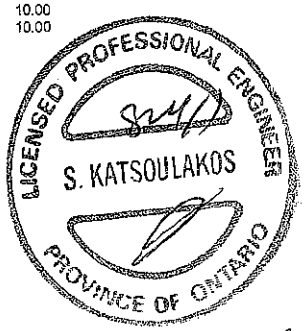
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO			
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	G-C	-129 / 102	0.09 (1)	
B-C	-210 / 0	-84.3	-84.3	0.20 (1)	6.25	B-G	0 / 223	0.05 (1)	
C-D	-210 / 0	-84.3	-84.3	0.20 (1)	6.25	G-D	0 / 223	0.05 (1)	
D-E	0 / 32	-84.3	-84.3	0.11 (1)	10.00				
H-B	-551 / 0	0.0	0.0	0.13 (1)	7.81				
F-D	-551 / 0	0.0	0.0	0.13 (1)	7.81				
H-G	0 / 0	-28.0	-28.0	0.15 (3)	10.00				
G-F	0 / 0	-28.0	-28.0	0.15 (3)	10.00				



DWG NO. TAM 4/844/17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.29")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.29")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.20 (C-D:1) , BC=0.15 (F-G:3) , WB=0.09 (C-G:1) , SSI=0.12 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.36 (B) (INPUT = 0.90)
JSI METAL= 0.09 (D) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285414	T34	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Wed Aug 23 09:35:32 2017 Page 1
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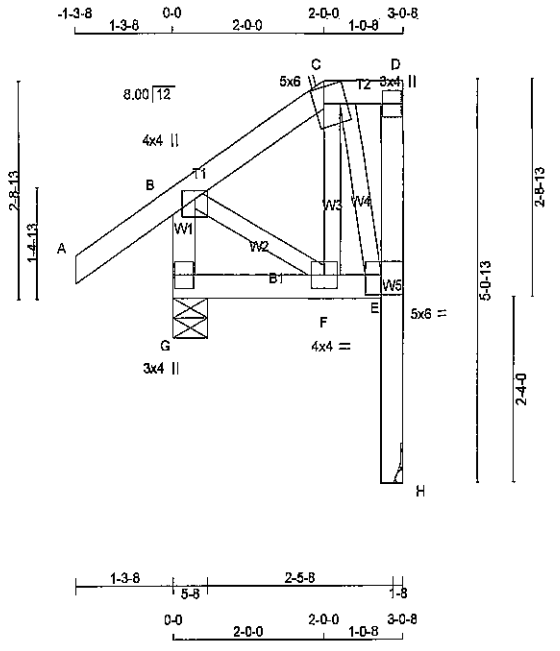


TOTAL WEIGHT = 2 X 21 = 41 lb

(M)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
H - D	2x4	DRY	No.2	SPF
G - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTWW+m	MT20	5.0	6.0	2.50	1.50
D	TMV+p	MT20	3.0	4.0		
E	BVMW-I	MT20	5.0	6.0	3.00	2.50
F	BMWW-t	MT20	4.0	4.0		
G	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	171	0	171	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
G	286	0	286	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	140	78 / 0	32 / 0	0 / 0	0 / 0	30 / 0	0 / 0
G	219	148 / 0	32 / 0	0 / 0	0 / 0	39 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

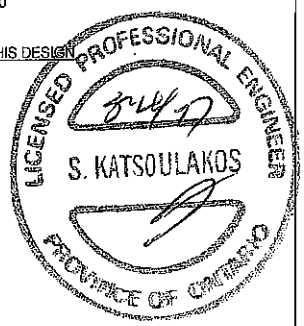
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.3	-84.3	0.11 (1)		10.00
B- C	-60 / 0	-84.3	-84.3	0.06 (1)		6.25
C- D	0 / 0	-84.3	-84.3	0.02 (1)		10.00
H- E	-171 / 0	0.0	0.0	0.03 (1)		7.81
E- D	-44 / 0	0.0	0.0	0.01 (1)		7.81
G- B	-259 / 0	0.0	0.0	0.03 (1)		7.81
G- F	0 / 0	-28.0	-28.0	0.03 (2)		10.00
F- E	0 / 51	-28.0	-28.0	0.03 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	0 / 82	0.02 (3)
C- E	-129 / 0	0.02 (1)
B- F	0 / 56	0.01 (1)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN.



DWG NO. TAM 4/1845-17
STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.11 (A-B:1) , BC=0.03 (F-G:2) , WB=0.02 (C-E:1) , SSI=0.08 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285414	T35	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Wed Aug 23 09:35:32 2017 Page 1

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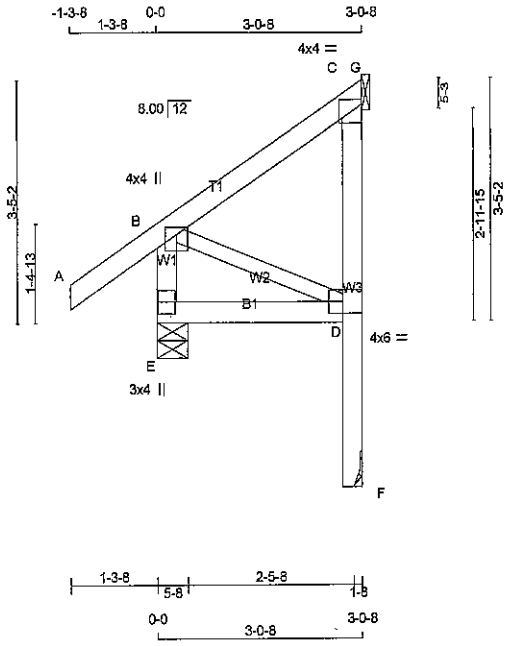

TOTAL WEIGHT = 19 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - G	2x4	DRY	No.2	SPF
F - C	2x4	DRY	No.2	SPF
E - B	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

BEARING NOTE: GAP BETWEEN INSIDE OF TOP CHORD BEARING AND FIRST DIAGONAL OR VERTICAL WEB SHALL NOT EXCEED 0.5 INCHES.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TMV1-p	MT20	4.0	4.0		Edge
D	BVMW-I	MT20	4.0	6.0	2.00	2.50
E	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
C	144	0	144	0	0	1-8	1-8 (** SEE "BEARING NOTE" **)
F	22	0	41	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
E	280	0	280	0	0	5-8	5-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	109	76 / 0	15 / 0	0 / 0	0 / 0	19 / 0	0 / 0
F	28	0 / -1	18 / 0	0 / 0	0 / 0	12 / 0	0 / 0
E	214	145 / 0	31 / 0	0 / 0	0 / 0	38 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	B- D	-1 / 0	0.00 (2)
B- C	0 / 2	-84.3	-84.3	0.12 (1)	10.00			
C- G	0 / 0	0.0	0.0	0.00 (1)	10.00			
F- D	-41 / 0	0.0	0.0	0.01 (3)	7.81			
D- C	0 / 34	0.0	0.0	0.01 (3)	10.00			
E- B	-238 / 0	0.0	0.0	0.02 (1)	7.81			
E- D	0 / 0	-28.0	-28.0	0.08 (3)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN.

DWG NO. TAM 4/184617
STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.12 (B-C:1) , BC=0.08 (D-E:3) , WB=0.00 (B-D:2) , SSI=0.08 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90)
JSI METAL= 0.03 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285414	T36C	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Wed Aug 23 09:35:33 2017 Page 1

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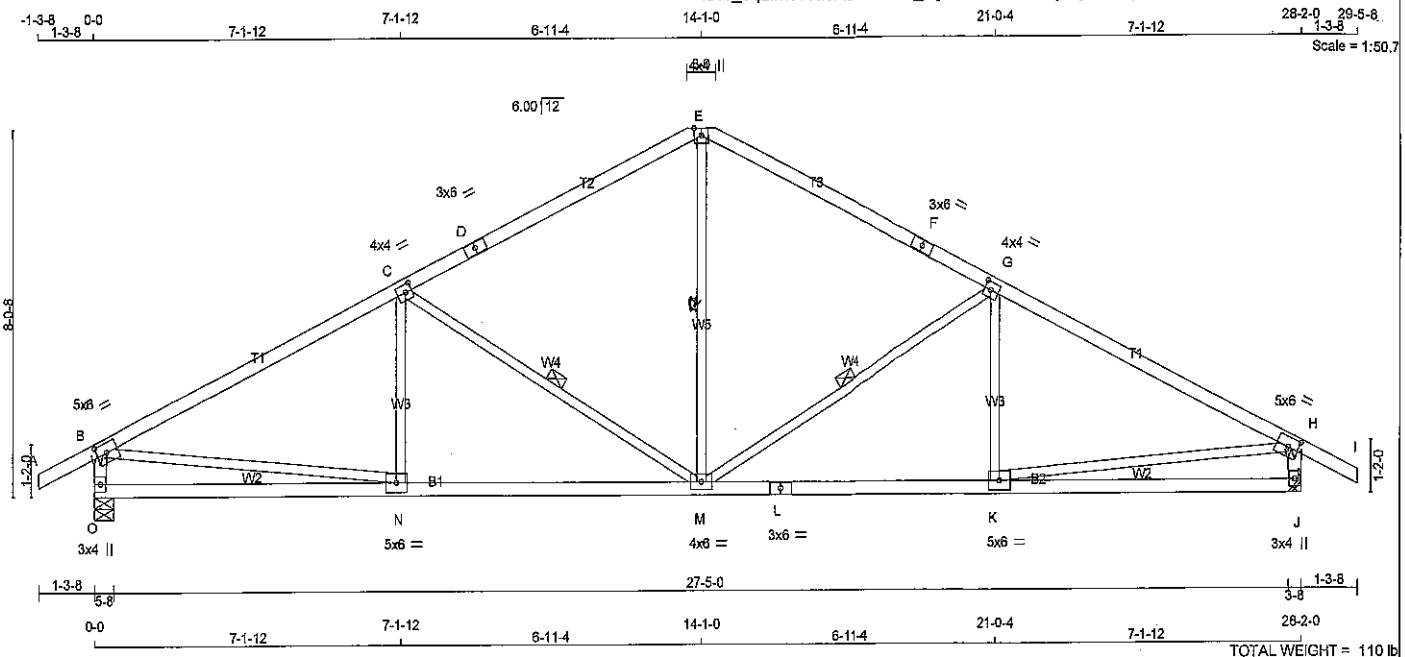

TOTAL WEIGHT = 110 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	6.0		
E	TMTMW+p	MT20	4.0	4.0	Edge	
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
H	TMVW-t	MT20	5.0	6.0	2.50	2.75
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
O	1695	0	1695	0	0	5-8	5-8
J	1695	0	1695	0	0	HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1376	790 / 0	296 / 0	0 / 0	0 / 0	290 / 0	0 / 0
J	1376	790 / 0	296 / 0	0 / 0	0 / 0	290 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.80 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-M, C-M. E-M

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS		
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 26	-84.3	-84.3	0.11 (1)	10.00	M- E	0 / 982	0.22 (1)
B- C	-2254 / 0	-84.3	-84.3	0.70 (1)	3.80	M- G	-738 / 0	0.34 (1)
C- D	-1622 / 0	-84.3	-84.3	0.61 (1)	4.45	K- G	-27 / 269	0.06 (3)
D- E	-1622 / 0	-84.3	-84.3	0.61 (1)	4.45	C- M	-738 / 0	0.34 (1)
E- F	-1622 / 0	-84.3	-84.3	0.61 (1)	4.45	N- C	-27 / 269	0.06 (3)
F- G	-1622 / 0	-84.3	-84.3	0.61 (1)	4.45	B- N	0 / 2059	0.46 (1)
G- H	-2254 / 0	-84.3	-84.3	0.70 (1)	3.80	K- H	0 / 2059	0.46 (1)
H- I	0 / 26	-84.3	-84.3	0.11 (1)	10.00			
O- B	-1613 / 0	0.0	0.0	0.16 (1)	6.52			
J- H	-1613 / 0	0.0	0.0	0.16 (1)	6.52			
O- N	0 / 0	-28.0	-28.0	0.34 (3)	10.00			
N- M	0 / 2044	-28.0	-28.0	0.57 (2)	10.00			
M- L	0 / 2044	-28.0	-28.0	0.57 (2)	10.00			
L- K	0 / 2044	-28.0	-28.0	0.57 (2)	10.00			
K- J	0 / 0	-28.0	-28.0	0.34 (3)	10.00			

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.94")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.94")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.70 (B-C:1) , BC=0.57 (M-N:2) , WB=0.46 (B-N:1) , SSI=0.26 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (N) (INPUT = 0.90)
JSI METAL= 0.61 (B) (INPUT = 1.00)

DWG NO. TAM 4/1648 17
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285414	T36	30	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Wed Aug 23 09:35:33 2017 Page 1
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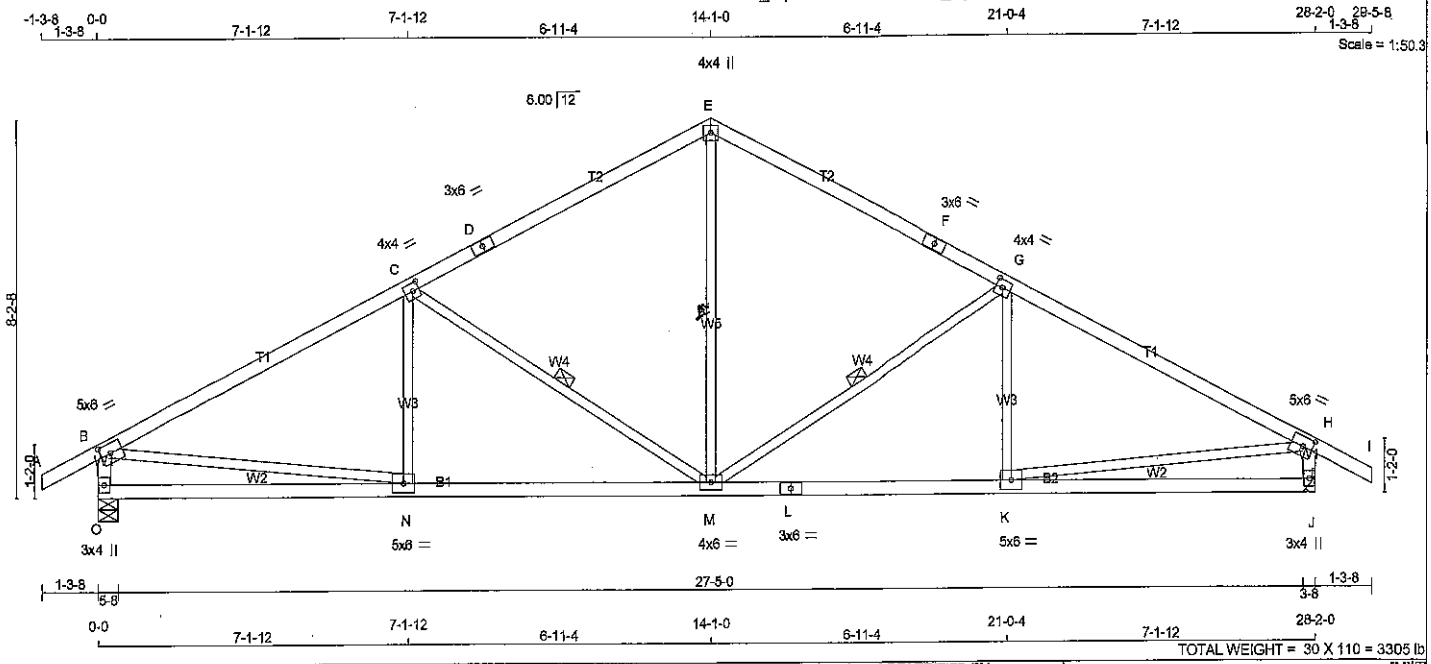


TOTAL WEIGHT = 30 X 110 = 3305 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	6.0		
E	TTW+p	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
H	TMVW-t	MT20	5.0	6.0	2.50	2.75
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1695	0	1695	0	0	5-8	5-8
J	1695	0	1695	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1376	790 / 0	296 / 0	0 / 0	0 / 0	290 / 0	0 / 0
J	1376	790 / 0	296 / 0	0 / 0	0 / 0	290 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.80 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-M, C-M. EM

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 26	-84.3	-84.3	0.11 (1)	10.00
B- C	-2254 / 0	-84.3	-84.3	0.70 (1)	3.80
C- D	-1622 / 0	-84.3	-84.3	0.61 (1)	4.45
D- E	-1622 / 0	-84.3	-84.3	0.61 (1)	4.45
E- F	-1622 / 0	-84.3	-84.3	0.61 (1)	4.45
F- G	-1622 / 0	-84.3	-84.3	0.61 (1)	4.45
G- H	-2254 / 0	-84.3	-84.3	0.70 (1)	3.80
H- I	0 / 26	-84.3	-84.3	0.11 (1)	10.00
O- B	-1613 / 0	0.0	0.0	0.16 (1)	6.52
J- H	-1613 / 0	0.0	0.0	0.16 (1)	6.52
O- N	0 / 0	-28.0	-28.0	0.34 (3)	10.00
N- M	0 / 2044	-28.0	-28.0	0.57 (2)	10.00
M- L	0 / 2044	-28.0	-28.0	0.57 (2)	10.00
L- K	0 / 2044	-28.0	-28.0	0.57 (2)	10.00
K- J	0 / 0	-28.0	-28.0	0.34 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- E	0 / 982	0.22 (1)
M- G	-738 / 0	0.34 (1)
K- G	-27 / 269	0.06 (3)
C- M	-738 / 0	0.34 (1)
N- C	-27 / 269	0.06 (3)
B- N	0 / 2059	0.46 (1)
K- H	0 / 2059	0.46 (1)

[M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.94")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.94")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.70 (B-C:1) , BC=0.57 (M-N:2) , WB=0.46 (B-N:1) , SSI=0.26 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

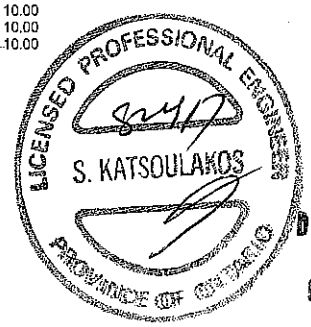
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (N) (INPUT = 0.90)
JSI METAL= 0.61 (B) (INPUT = 1.00)



DWG NO. TAM41042-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285414	T38S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Wed Aug 23 09:35:33 2017 Page 1

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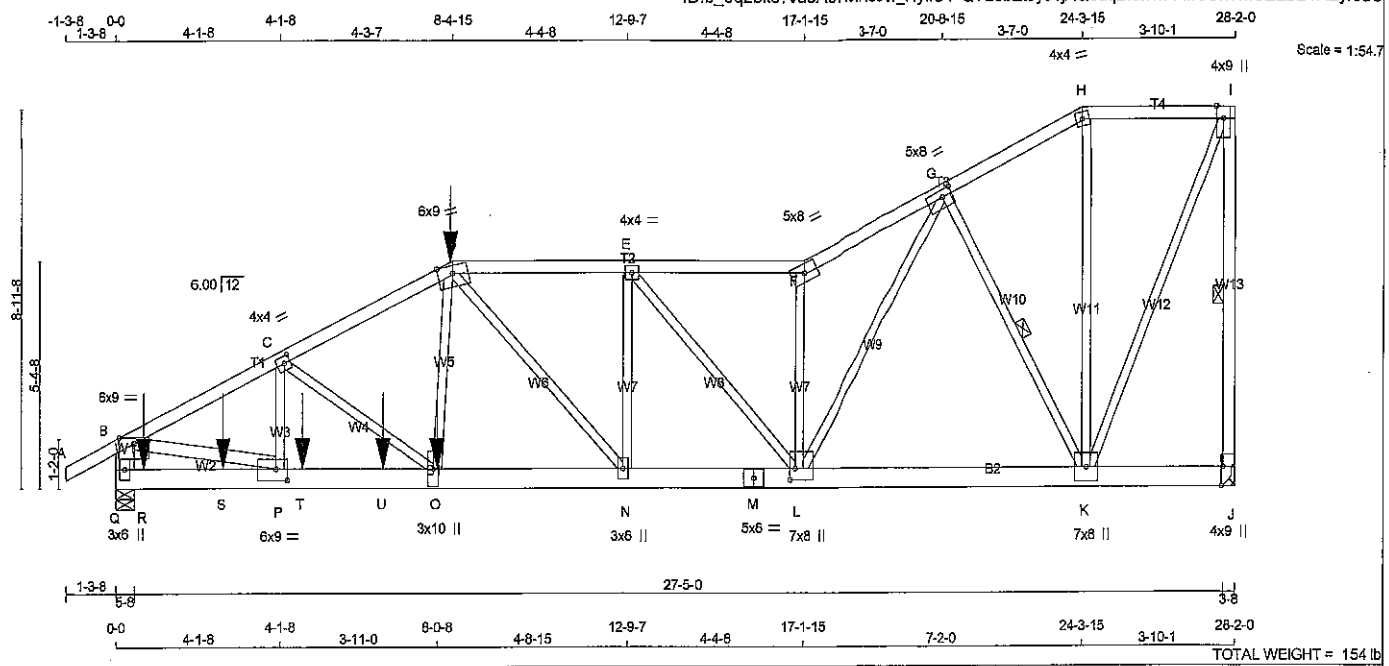


TOTAL WEIGHT = 154 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - I	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
Q - B	2x6	DRY	No.2	SPF
Q - M	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
B - P	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	6.0	9.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	6.0	9.0	Edge	
E	TMWW-t	MT20	4.0	4.0		
F	TTW-h	MT20	5.0	8.0		
G	TMWW-t	MT20	5.0	8.0	2.00	3.00
H	TTW-m	MT20	4.0	4.0		
I	TMVW+p	MT20	4.0	9.0	Edge	
J	BMV1+t	MT20	4.0	9.0	Edge	0.50
K	BMWWW+t	MT20	7.0	8.0		
L	BMWWW+t	MT20	7.0	8.0	3.25	1.50
M	BS-t	MT20	5.0	6.0		
N	BMWW+t	MT20	3.0	6.0		
O	BMWW+t	MT20	3.0	10.0		
P	BMWW-t	MT20	6.0	9.0	3.25	3.50
Q	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 518.8 lbs FACTORED DOWN AT 8-4-15 ON TOP CHORD, AND 61.0 lbs FACTORED DOWN AT 8-4, 73.3 lbs FACTORED DOWN AT 2-8-4, 73.3 lbs FACTORED DOWN AT 4-8-4, AND 73.3 lbs FACTORED DOWN AT 6-8-4, AND 1582.7 lbs FACTORED DOWN AT 8-0-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
J	2209	0	2209	0	0	IN-SX	IN-SX
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-8	
Q	3341	0	3341	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	1802	1019 / 0	398 / 0	0 / 0	0 / 0	385 / 0	0 / 0
Q	2742	1523 / 0	624 / 0	0 / 0	0 / 0	595 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.62 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF I-J, G-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 26	-84.3	-84.3	0.12 (1)	10.00	P-C	-636 / 0	0.12 (1)	
B-C	-4792 / 0	-84.3	-84.3	0.61 (1)	2.72	C-O	0 / 211	0.05 (1)	
C-D	-5016 / 0	-84.3	-84.3	0.66 (1)	2.62	O-D	0 / 1817	0.40 (1)	
D-E	-4184 / 0	-84.3	-84.3	0.60 (1)	2.88	D-N	-245 / 0	0.20 (1)	
E-F	-3522 / 0	-84.3	-84.3	0.51 (1)	3.24	N-E	0 / 340	0.08 (2)	
F-G	-3962 / 0	-84.3	-84.3	0.49 (1)	3.06	E-L	-1020 / 0	0.79 (1)	
G-H	-1002 / 0	-84.3	-84.3	0.25 (1)	5.89	L-F	-2030 / 0	0.84 (1)	
H-I	-880 / 0	-84.3	-84.3	0.25 (1)	6.18	L-G	0 / 3351	0.83 (1)	
J-I	-2182 / 0	0.0	0.0	0.80 (1)	4.56	G-K	-2269 / 0	0.92 (1)	
Q-B	-3212 / 0	0.0	0.0	0.23 (1)	5.86	K-H	0 / 218	0.05 (2)	
						K-I	0 / 2206	0.55 (1)	
Q-R	0 / 0	-28.0	-28.0	0.12 (2)	10.00	B-P	0 / 2206	0.55 (1)	
R-S	0 / 0	-28.0	-28.0	0.12 (2)	10.00				
S-P	0 / 0	-28.0	-28.0	0.12 (2)	10.00				
P-T	0 / 4300	-28.0	-28.0	0.67 (1)	10.00				
T-U	0 / 4300	-28.0	-28.0	0.67 (1)	10.00				
U-O	0 / 4300	-28.0	-28.0	0.67 (1)	10.00				
O-N	0 / 4346	-28.0	-28.0	0.63 (1)	10.00				
N-M	0 / 4184	-28.0	-28.0	0.59 (1)	10.00				
M-L	0 / 4184	-28.0	-28.0	0.59 (1)	10.00				
L-K	0 / 1956	-28.0	-28.0	0.37 (1)	10.00				
K-J	0 / 0	-28.0	-28.0	0.13 (3)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
D	8-4-15	-46	-52	---	FRONT	VERT	DEAD
D	8-4-15	-473	-473	---	FRONT	VERT	SNOW
O	8-0-8	-1583	-1583	---	BACK	VERT	TOTAL
R	8-4	-46	-81	---	BACK	VERT	TOTAL
S	2-8-4	-42	-73	---	BACK	VERT	TOTAL
T	4-8-4	-42	-73	---	BACK	VERT	TOTAL
U	6-8-4	-42	-73	---	BACK	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.94")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.23")
ALLOWABLE DEFL.(TL)= L/360 (0.94")
CALCULATED VERT. DEFL.(TL) = L/ 911 (0.37")

CSI: TC=0.80 (I-J:1) , BC=0.67 (O-P:1) , WB=0.92 (G-K:1) , SSI=0.20 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

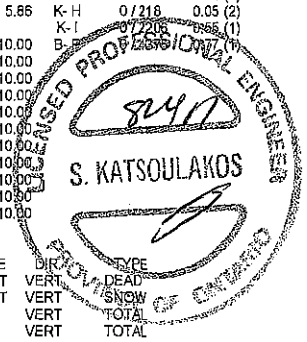
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (P) (INPUT = 0.90)
JSI METAL= 0.72 (P) (INPUT = 1.00)



DWG NO. TAM 4/1849-17

STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285414	T39S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Wed Aug 23 09:35:34 2017 Page 1
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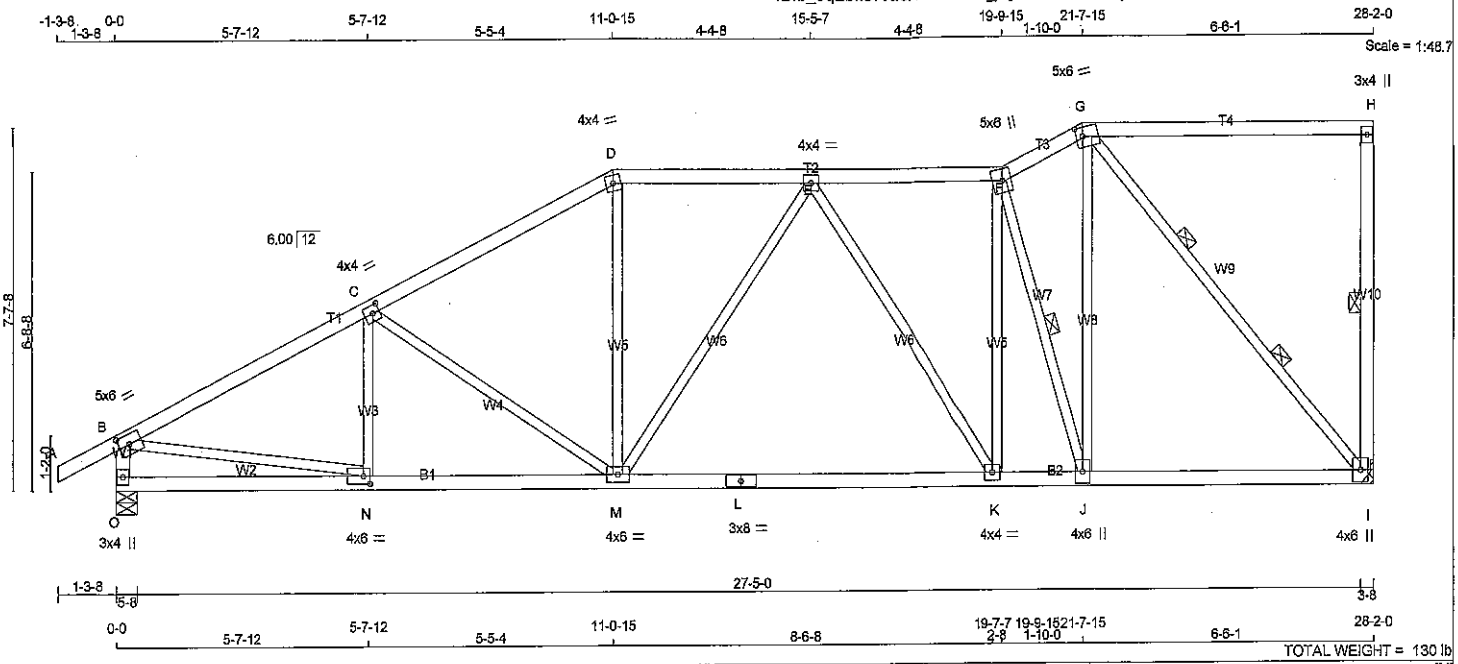


TOTAL WEIGHT = 130 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTWW+m	MT20	5.0	6.0		
G	TTWW-m	MT20	5.0	6.0	2.25	1.75
H	TMV+p	MT20	3.0	4.0		
I	BMVW1+p	MT20	4.0	8.0		
J	BMWW+t	MT20	4.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	8.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BMWW-t	MT20	4.0	6.0	2.00	1.75
O	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
I	1581	0	1581	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	
O	1695	0	1695	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1298	721 / 0	296 / 0	0 / 0	0 / 0	282 / 0	0 / 0
O	1378	790 / 0	296 / 0	0 / 0	0 / 0	290 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.97 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF H-I, F-J.
2 LATERAL BRACE(S) AT 1/3 LENGTH OF G-I.

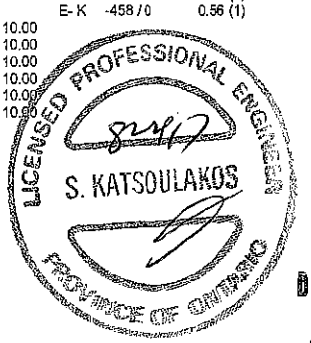
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 26	-84.3	-84.3	0.11 (1)	10.00
B- C	-2250 / 0	-84.3	-84.3	0.58 (1)	3.97
C- D	-1899 / 0	-84.3	-84.3	0.53 (1)	4.29
D- E	-1684 / 0	-84.3	-84.3	0.31 (1)	4.79
E- F	-1475 / 0	-84.3	-84.3	0.30 (1)	5.05
F- G	-1201 / 0	-84.3	-84.3	0.06 (1)	5.78
G- H	0 / 0	-84.3	-84.3	0.61 (1)	10.00
I- H	-274 / 0	0.0	0.0	0.07 (1)	6.25
O- B	-1625 / 0	0.0	0.0	0.16 (1)	6.49
O- N	0 / 0	-28.0	-28.0	0.19 (2)	10.00
N- M	0 / 2034	-28.0	-28.0	0.57 (2)	10.00
M- L	0 / 1729	-28.0	-28.0	0.54 (2)	10.00
L- K	0 / 1729	-28.0	-28.0	0.54 (2)	10.00
K- J	0 / 1466	-28.0	-28.0	0.49 (2)	10.00
J- I	0 / 1094	-28.0	-28.0	0.37 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- C	-161 / 118	0.04 (1)
C- M	-430 / 0	0.35 (1)
M- D	0 / 522	0.12 (1)
K- F	0 / 539	0.12 (1)
F- J	-1355 / 0	0.42 (1)
J- G	0 / 1418	0.32 (1)
G- I	-1648 / 0	0.98 (1)
B- N	0 / 2057	0.46 (1)
M- E	-80 / 74	0.10 (1)
E- K	-458 / 0	0.56 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.94")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.20")
ALLOWABLE DEFL.(TL)= L/360 (0.94")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.33")

CSI: TC=0.61 (G-H:1) , BC=0.57 (M-N:2) , WB=0.98 (G-I:1) , SSI=0.21 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (G) (INPUT = 0.90)
JSI METAL= 0.65 (L) (INPUT = 1.00)

DWG NO. TAM 4/ESO-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285414	T40	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Wed Aug 23 09:35:34 2017 Page 1

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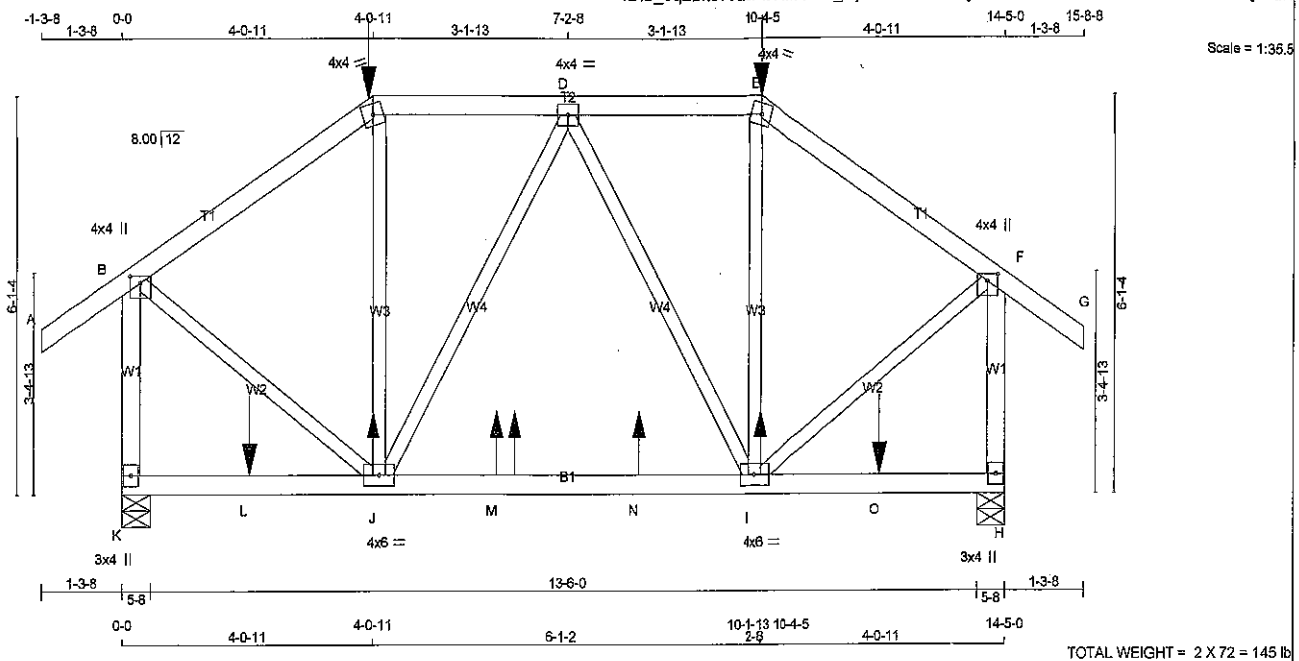

TOTAL WEIGHT = 2 X 72 = 145 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTW-m	MT20	4.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TTW-m	MT20	4.0	4.0		
F	TMVW+p	MT20	4.0	4.0	1.25	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 101.5 lbs FACTORED DOWN AT 4-0-11, AND 101.5 lbs FACTORED DOWN AT 10-4-5 ON TOP CHORD, AND 110.7 lbs FACTORED DOWN AT 2-0-12, 1.0 lbs FACTORED UP AT 4-0-12, 1.0 lbs FACTORED UP AT 6-0-12, 1.0 lbs FACTORED UP AT 8-4-4, 1.0 lbs FACTORED UP AT 8-4-4, AND 1.0 lbs FACTORED UP AT 10-4-4, AND 110.7 lbs FACTORED DOWN AT 12-4-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DWG NO. TAM4/05/-17
STRUCTURAL
COMPONENT ONLY

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
K	1134	0	1134	0	0	5-8	5-8
H	1135	0	1135	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	893	557 / 0	161 / 0	0 / 0	0 / 0	174 / 0	0 / 0
H	893	558 / 0	161 / 0	0 / 0	0 / 0	174 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 32	-84.3	-84.3	0.12 (1)	10.00	J- C	-16 / 130	0.03 (3)
B- C	-647 / 0	-84.3	-84.3	0.27 (1)	6.25	I- E	-16 / 130	0.03 (3)
C- D	-537 / 0	-84.3	-84.3	0.15 (1)	6.25	B- J	0 / 668	0.17 (1)
D- E	-537 / 0	-84.3	-84.3	0.15 (1)	6.25	I- F	0 / 668	0.17 (1)
E- F	-647 / 0	-84.3	-84.3	0.27 (1)	6.25	J- D	-183 / 0	0.15 (1)
F- G	0 / 32	-84.3	-84.3	0.12 (1)	10.00	D- I	-182 / 0	0.15 (1)
K- B	-1044 / 0	0.0	0.0	0.21 (1)	7.66			
H- F	-1044 / 0	0.0	0.0	0.21 (1)	7.66			
K- L	0 / 0	-28.0	-28.0	0.22 (2)	10.00			
L- J	0 / 0	-28.0	-28.0	0.22 (2)	10.00			
J- M	0 / 622	-28.0	-28.0	0.29 (2)	10.00			
M- N	0 / 622	-28.0	-28.0	0.29 (2)	10.00			
N- I	0 / 622	-28.0	-28.0	0.29 (2)	10.00			
I- O	0 / 0	-28.0	-28.0	0.22 (2)	10.00			
O- H	0 / 0	-28.0	-28.0	0.22 (2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	4-0-11	-10	-11	---	FRONT	VERT	DEAD
C	4-0-11	-92	-92	---	FRONT	VERT	SNOW
E	10-4-5	-10	-11	---	FRONT	VERT	DEAD
E	10-4-5	-92	-92	---	FRONT	VERT	SNOW
I	10-4-4	1	---	1	FRONT	VERT	TOTAL
J	4-0-12	1	---	1	FRONT	VERT	TOTAL
L	2-0-12	-111	-111	---	FRONT	VERT	TOTAL
M	6-0-12	1	---	1	FRONT	VERT	TOTAL
M	8-4-4	1	---	1	FRONT	VERT	TOTAL
N	8-4-4	1	---	1	FRONT	VERT	TOTAL
O	12-4-4	-111	-111	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.48")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.48")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.27 (E-F:1) , BC=0.29 (I-J:2) , WB=0.17 (F-I:1) , SSI=0.14 (H-I:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (I) (INPUT = 0.90)
JSI METAL= 0.19 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285425	T40A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Dec 12 2017 MiTek Industries, Inc. Tue Feb 6 11:11:32 2018 Page 1

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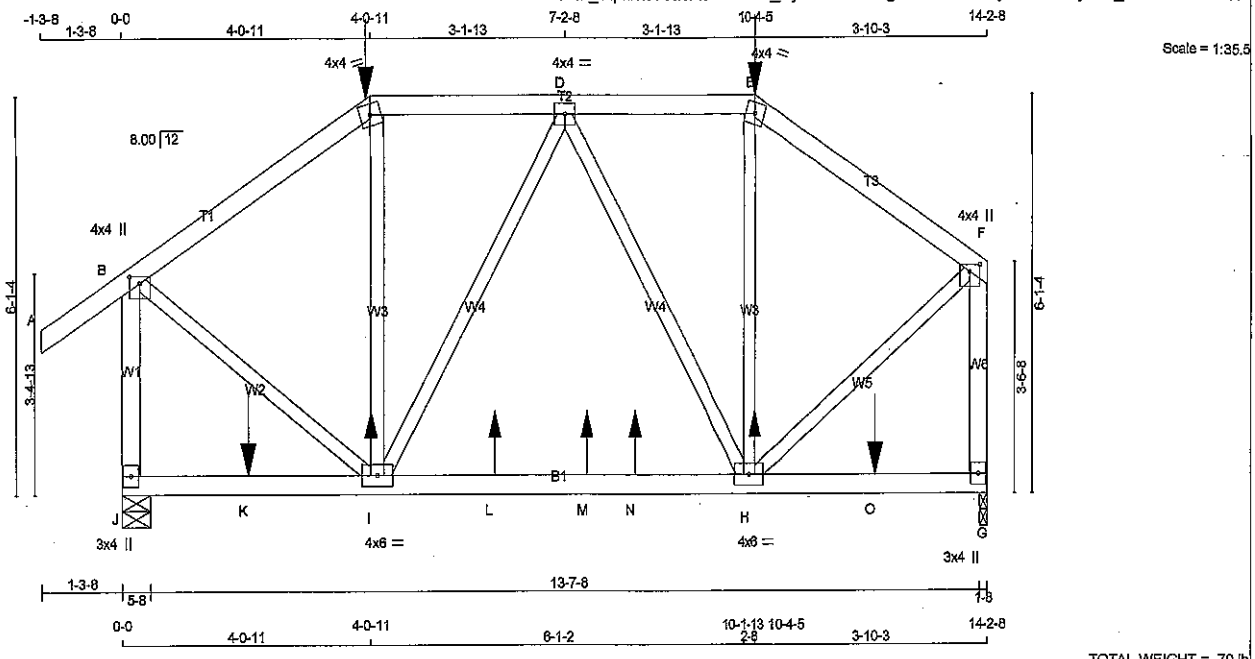

TOTAL WEIGHT = 70 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTW-m	MT20	4.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TTW-m	MT20	4.0	4.0		
F	TMVW+p	MT20	4.0	4.0	1.25	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	6.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMV1+p	MT20	3.0	4.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 101.5 lbs FACTORED DOWN AT 4-0-11, AND 101.5 lbs FACTORED DOWN AT 10-4-5 ON TOP CHORD, AND 110.7 lbs FACTORED DOWN AT 2-0-12, 1.0 lbs FACTORED DOWN AND 1.0 lbs FACTORED UP AT 4-0-12, 1.0 lbs FACTORED DOWN AND 1.0 lbs FACTORED UP AT 6-0-12, 1.0 lbs FACTORED DOWN AND 1.0 lbs FACTORED UP AT 7-6-12, 1.0 lbs FACTORED DOWN AND 1.0 lbs FACTORED UP AT 8-4-4, AND 1.0 lbs FACTORED DOWN AND 1.0 lbs FACTORED UP AT 10-4-4, AND 110.7 lbs FACTORED DOWN AT 12-4-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1120	0	1120	0	0	5-8	5-8
G	1010	0	1010	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	861	550 / 0	159 / 0	0 / 0	0 / 0	172 / 0	0 / 0
G	807	484 / 0	159 / 0	0 / 0	0 / 0	164 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 32	-84.3	-84.3	0.12 (1)	10.00		I- C	-23 / 127	0.03 (3)
B- C	-634 / 0	-84.3	-84.3	0.27 (1)	6.25		H- E	-25 / 123	0.03 (3)
C- D	-526 / 0	-84.3	-84.3	0.15 (1)	6.25		B- I	0 / 656	0.16 (1)
D- E	-510 / 0	-84.3	-84.3	0.15 (1)	6.25		H- F	0 / 658	0.16 (1)
E- F	-614 / 0	-84.3	-84.3	0.25 (1)	6.25		I- D	-164 / 0	0.13 (1)
J- B	-1030 / 0	0.0	0.0	0.21 (1)	7.71		D- H	-201 / 0	0.16 (1)
G- F	-920 / 0	0.0	0.0	0.20 (1)	7.81				
J- K	0 / 0	-28.0	-28.0	0.22 (2)	10.00				
K- I	0 / 0	-28.0	-28.0	0.22 (2)	10.00				
I- L	0 / 603	-28.0	-28.0	0.29 (2)	10.00				
L- M	0 / 603	-28.0	-28.0	0.29 (2)	10.00				
M- N	0 / 603	-28.0	-28.0	0.29 (2)	10.00				
N- H	0 / 603	-28.0	-28.0	0.29 (2)	10.00				
H- O	0 / 0	-28.0	-28.0	0.21 (2)	10.00				
O- G	0 / 0	-28.0	-28.0	0.21 (2)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	4-0-11	-10	-11	---	FRONT	VERT	DEAD
C	4-0-11	-92	-92	---	FRONT	VERT	SNOW
E	10-4-5	-10	-11	---	FRONT	VERT	DEAD
E	10-4-5	-92	-92	---	FRONT	VERT	SNOW
H	10-4-4	1	1	1	FRONT	VERT	TOTAL
I	4-0-12	1	1	1	FRONT	VERT	TOTAL
K	2-0-12	-111	-111	---	FRONT	VERT	TOTAL
L	6-0-12	1	1	1	FRONT	VERT	TOTAL
M	7-6-12	1	1	1	FRONT	VERT	TOTAL
N	8-4-4	1	1	1	FRONT	VERT	TOTAL
O	12-4-4	-111	-111	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.47")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.47")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.27/1.00 (B-C:1) , BC=0.29/1.00 (H-I:2) , WB=0.16/1.00 (F-H:1) , SSI=0.14/1.00 (I-J:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JS GRIP= 0.82 (H) (INPUT = 0.90)
JS METAL= 0.19 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 8093 -R8
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285414	T41	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Wed Aug 23 09:35:35 2017 Page 1

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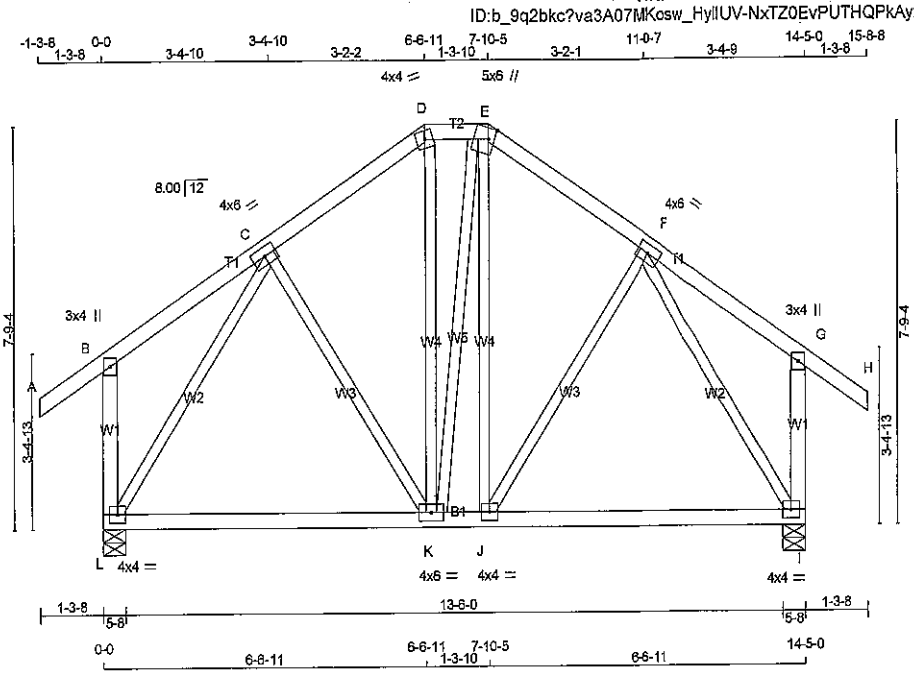


TOTAL WEIGHT = 2 X 83 = 167 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.50	1.50
F	TMWW-t	MT20	4.0	6.0		
G	TMV+p	MT20	3.0	4.0		
I	BMWW1-t	MT20	4.0	4.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	925	0	925	0	0	5-8	5-8
I	925	0	925	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	743	439 / 0	151 / 0	0 / 0	0 / 0	152 / 0	0 / 0
I	743	439 / 0	151 / 0	0 / 0	0 / 0	152 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, I

BRACING

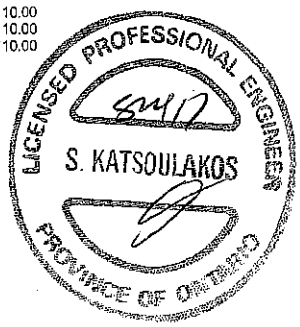
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	C- K	0 / 106	0.02 (3)
B- C	0 / 18	-84.3	-84.3	0.15 (1)	10.00	K- D	0 / 144	0.03 (2)
C- D	-503 / 0	-84.3	-84.3	0.11 (1)	6.25	K- E	0 / 7	0.00 (2)
D- E	-404 / 0	-84.3	-84.3	0.02 (1)	6.25	J- E	0 / 137	0.03 (2)
E- F	-502 / 0	-84.3	-84.3	0.11 (1)	6.25	J- F	0 / 105	0.02 (3)
F- G	0 / 18	-84.3	-84.3	0.15 (1)	10.00	L- C	-740 / 0	0.49 (1)
G- H	0 / 32	-84.3	-84.3	0.11 (1)	10.00	F- I	-740 / 0	0.48 (1)
L- B	-221 / 0	0.0	0.0	0.04 (1)	7.81			
I- G	-221 / 0	0.0	0.0	0.04 (1)	7.81			
L- K	0 / 390	-28.0	-28.0	0.34 (2)	10.00			
K- J	0 / 403	-28.0	-28.0	0.35 (2)	10.00			
J- I	0 / 390	-28.0	-28.0	0.33 (2)	10.00			



DWG NO. TAM 41853-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.48")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.48")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.15 (F-G:1) , BC=0.35 (J-K:2) , WB=0.49 (C-L:1) , SSI=0.15 (K-L:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (C) (INPUT = 0.90)
JSI METAL= 0.17 (C) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285425	T41A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Dec 12 2017 MiTek Industries, Inc. Tue Feb 6 11:11:33 2018 Page 1
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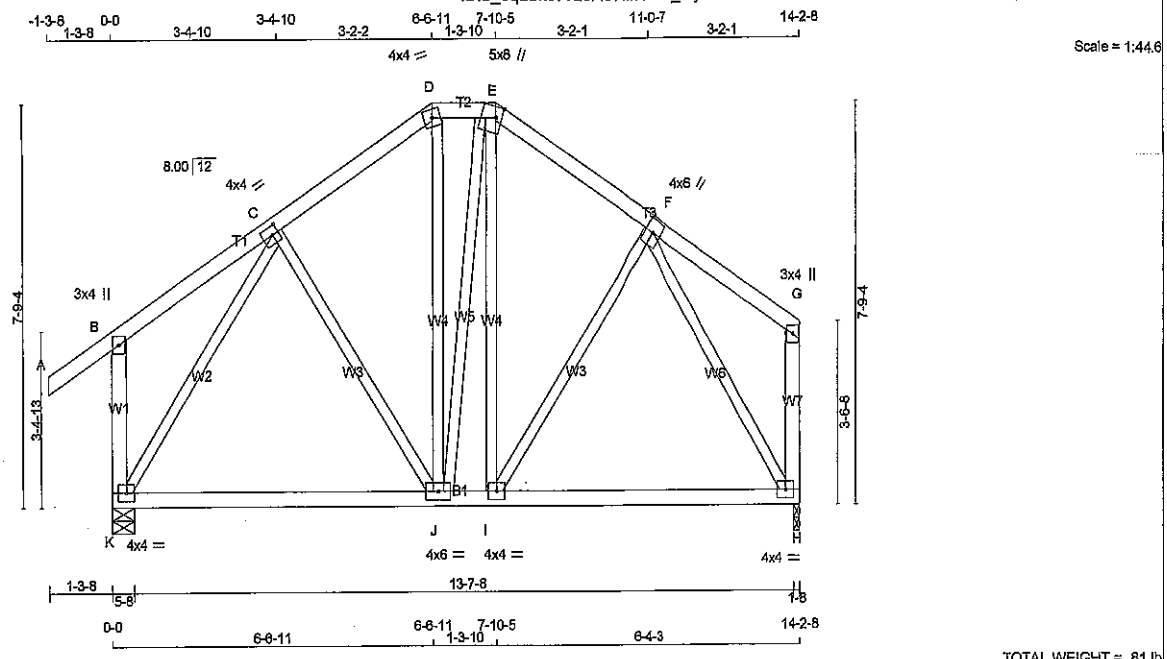


TOTAL WEIGHT = 81 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTW-m	MT20	4.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.50	1.50
F	TMWW+t	MT20	4.0	6.0		
G	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	913	0	913	0	0	5-8	5-8
H	797	0	797	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	733	434 / 0	149 / 0	0 / 0	0 / 0	150 / 0	0 / 0
H	655	364 / 0	149 / 0	0 / 0	0 / 0	142 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00
B- C	0 / 18	-84.3	-84.3	0.15 (1)	10.00
C- D	-490 / 0	-84.3	-84.3	0.11 (1)	6.25
D- E	-394 / 0	-84.3	-84.3	0.02 (1)	6.25
E- F	-485 / 0	-84.3	-84.3	0.11 (1)	6.25
F- G	0 / 18	-84.3	-84.3	0.14 (1)	10.00
K- B	-221 / 0	0.0	0.0	0.04 (1)	7.81
H- G	-96 / 0	0.0	0.0	0.02 (1)	7.81
K- J	0 / 383	-28.0	-28.0	0.34 (2)	10.00
J- I	0 / 390	-28.0	-28.0	0.35 (2)	10.00
I- H	0 / 384	-28.0	-28.0	0.31 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- J	0 / 103	0.02 (3)
J- D	0 / 139	0.03 (2)
J- E	0 / 29	0.01 (2)
I- E	0 / 108	0.02 (2)
I- F	0 / 111	0.03 (3)
K- C	-727 / 0	0.48 (1)
F- H	-726 / 0	0.48 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.47")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.47")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.15/1.00 (B-C:1) , BC=0.35/1.00 (I-J:2) , WB=0.48/1.00 (C-K:1) , SSI=0.15/1.00 (J-K:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

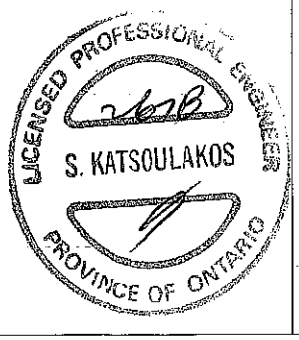
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.25 (C) (INPUT = 1.00)



DWG NO. TAM B094 -R
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285414	T42	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Wed Aug 23 09:35:35 2017 Page 1

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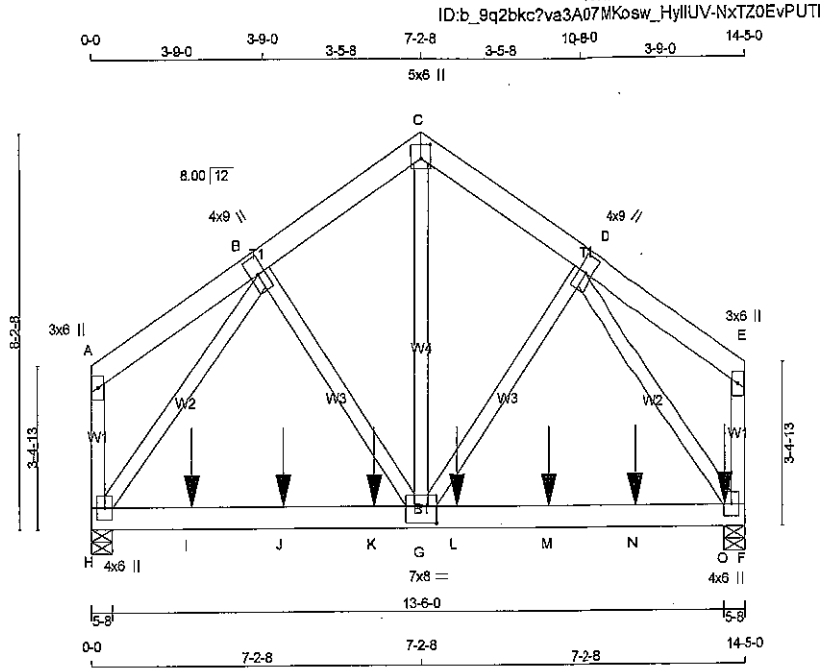


TOTAL WEIGHT = 3 X 99 = 297 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
H - F	2x6	DRY	1650F 1.5E	SPF
ALL WEBS	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	TOP
C- E	2	12	TOP
H- A	1	4	TOP
F- E	1	4	SIDE(54.6)
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
H- F	2	6	SIDE(840.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	6.0		
B	TMWW+t	MT20	4.0	9.0		
C	TTW+p	MT20	5.0	6.0	3.50	2.50
D	TMWW+t	MT20	4.0	9.0		
E	TMV+p	MT20	3.0	6.0		
F	BMVW1+p	MT20	4.0	6.0		
G	BMWWW-t	MT20	7.0	8.0	4.25	4.00
H	BMVW1+p	MT20	4.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1667.2 lbs FACTORED DOWN AT 2-2-4, 1667.2 lbs FACTORED DOWN AT 4-2-4, 1667.2 lbs FACTORED DOWN AT 6-2-4, 1552.9 lbs FACTORED DOWN AT 7-11-12, 1552.9 lbs FACTORED DOWN AT 9-11-12, AND 1552.9 lbs FACTORED DOWN AT 11-11-12, AND 1560.8 lbs FACTORED DOWN AT 13-11-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	5840	0	5840	0	0	5-8	5-8
F	7000	0	7000	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	4723	2738 / 0	998 / 0	0 / 0	0 / 0	987 / 0	0 / 0
F	5689	3252 / 0	1233 / 0	0 / 0	0 / 0	1204 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	-14 / 0	-84.3	-84.3	0.04 (1)	6.25	G- C	0 / 4736	0.25 (1)
B- C	-4566 / 0	-84.3	-84.3	0.05 (1)	6.25	G- D	0 / 1701	0.09 (1)
C- D	-4566 / 0	-84.3	-84.3	0.05 (1)	6.25	B- G	0 / 1701	0.09 (1)
D- E	-14 / 0	-84.3	-84.3	0.04 (1)	6.25	H- B	-5076 / 0	0.81 (1)
H- A	-169 / 0	0.0	0.0	0.01 (1)	7.81	D- F	-5076 / 0	0.81 (1)
F- E	-169 / 0	0.0	0.0	0.01 (1)	7.81			
H- I	0 / 2842	-28.0	-28.0	0.88 (1)	10.00			
I- J	0 / 2842	-28.0	-28.0	0.88 (1)	10.00			
J- K	0 / 2842	-28.0	-28.0	0.88 (1)	10.00			
K- G	0 / 2842	-28.0	-28.0	0.88 (1)	10.00			
G- L	0 / 2842	-28.0	-28.0	0.88 (1)	10.00			
L- M	0 / 2842	-28.0	-28.0	0.88 (1)	10.00			
M- N	0 / 2842	-28.0	-28.0	0.88 (1)	10.00			
N- O	0 / 2842	-28.0	-28.0	0.88 (1)	10.00			
O- F	0 / 2842	-28.0	-28.0	0.88 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
I	2-2-4	-1667	-1667	---	FRONT	VERT	TOTAL
J	4-2-4	-1667	-1667	---	FRONT	VERT	TOTAL
K	6-2-4	-1667	-1667	---	FRONT	VERT	TOTAL
L	7-11-12	-1553	-1553	---	FRONT	VERT	TOTAL
M	9-11-12	-1553	-1553	---	FRONT	VERT	TOTAL
N	11-11-12	-1553	-1553	---	FRONT	VERT	TOTAL
O	13-11-12	-1561	-1561	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.48")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.48")
CALCULATED VERT. DEFL.(TL) = L/ 777 (0.22")

CSI: TC=0.05 (C-D:1) , BC=0.88 (G-H:1) , WB=0.81 (D-F:1) , SSI=0.76 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

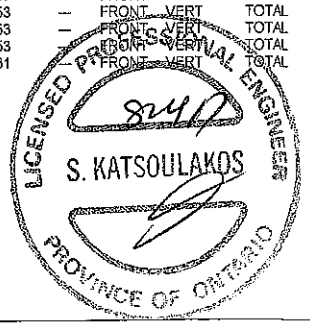
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (B) (INPUT = 0.90)
JSI METAL= 0.54 (D) (INPUT = 1.00)



DWG NO. TAM4/055-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285425	T42A	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Dec 12 2017 MiTek Industries, Inc. Tue Feb 6 11:11:33 2018 Page 1
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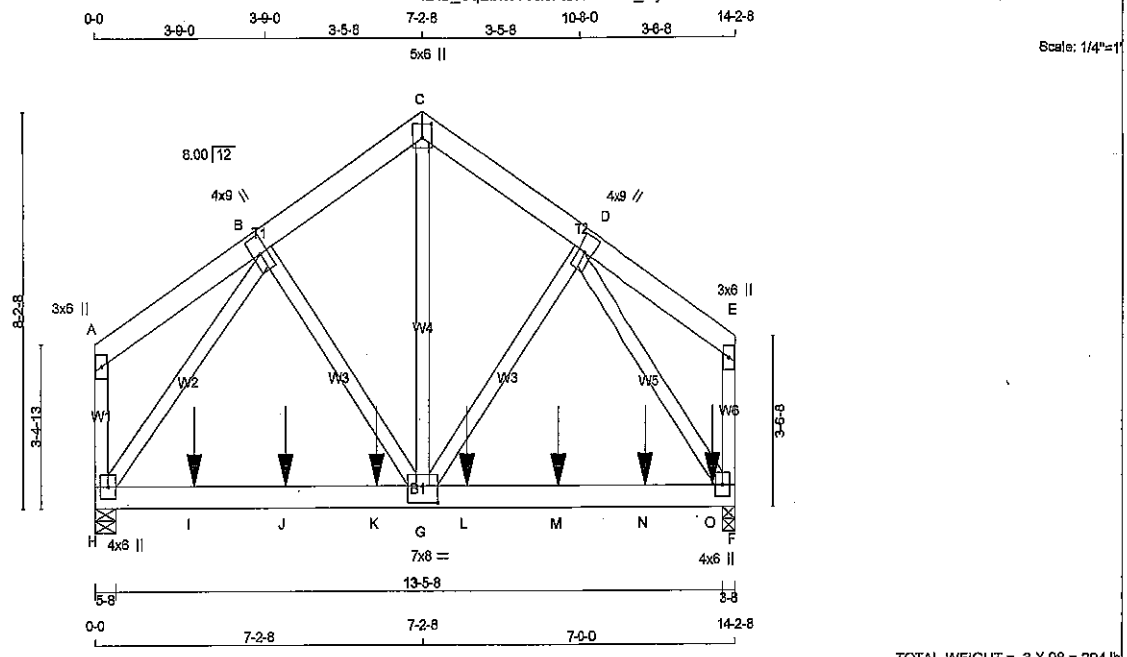

TOTAL WEIGHT = 3 X 98 = 294 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
H - F	2x6	DRY	2100F 1.8E	SPF
ALL WEBS	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	TOP
C- E	2	12	TOP
H- A	1	12	TOP
F- E	1	8	SIDE(146.9)
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
H- F	2	9	SIDE(617.5)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	6.0		
B	TMWW+t	MT20	4.0	9.0		
C	TTW+p	MT20	5.0	6.0	3.50	2.50
D	TMWW+t	MT20	4.0	9.0		
E	TMV+p	MT20	3.0	6.0		
F	BMVW1+p	MT20	4.0	6.0		
G	BMWWW-t	MT20	7.0	8.0	4.25	4.00
H	BMVW1+p	MT20	4.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1667.2 lbs FACTORED DOWN AT 2-2-4, 1552.9 lbs FACTORED DOWN AT 4-2-4, 1552.9 lbs FACTORED DOWN AT 6-2-4, 1552.9 lbs FACTORED DOWN AT 8-2-4, 1552.9 lbs FACTORED DOWN AT 10-2-4, AND 1552.9 lbs FACTORED DOWN AT 12-2-4, AND 1559.6 lbs FACTORED DOWN AT 13-8-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H	5555	0	5555	0	0	5-8	5-8
F	7031	0	7031	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	4511	2585 / 0	974 / 0	0 / 0	0 / 0	952 / 0	0 / 0
F	5725	3256 / 0	1253 / 0	0 / 0	0 / 0	1217 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	FORCE (LBS)	MAX CSI (LC)
A- B	-13 / 0	-84.3	-84.3	0.04 (1)	6.25	G- C	0 / 4438	0.24 (1)	
B- C	-4298 / 0	-84.3	-84.3	0.05 (1)	6.25	G- D	0 / 1738	0.09 (1)	
C- D	-4298 / 0	-84.3	-84.3	0.05 (1)	6.25	B- G	0 / 1585	0.08 (1)	
D- E	-14 / 0	-84.3	-84.3	0.03 (1)	6.25	H- B	-4789 / 0	0.76 (1)	
H- A	-165 / 0	0.0	0.0	0.01 (1)	7.81	D- F	-4832 / 0	0.74 (1)	
F- E	-160 / 0	0.0	0.0	0.01 (1)	7.81				
H- I	0 / 2682	-28.0	-28.0	0.66 (1)	10.00				
I- J	0 / 2682	-28.0	-28.0	0.66 (1)	10.00				
J- K	0 / 2682	-28.0	-28.0	0.66 (1)	10.00				
K- G	0 / 2682	-28.0	-28.0	0.66 (1)	10.00				
G- L	0 / 2594	-28.0	-28.0	0.66 (1)	10.00				
L- M	0 / 2594	-28.0	-28.0	0.66 (1)	10.00				
M- N	0 / 2594	-28.0	-28.0	0.66 (1)	10.00				
N- O	0 / 2594	-28.0	-28.0	0.66 (1)	10.00				
O- F	0 / 2594	-28.0	-28.0	0.66 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
I	2-2-4	-1667	-1667	—	BACK	VERT	TOTAL
J	4-2-4	-1553	-1553	—	BACK	VERT	TOTAL
K	6-2-4	-1553	-1553	—	BACK	VERT	TOTAL
L	8-2-4	-1553	-1553	—	BACK	VERT	TOTAL
M	10-2-4	-1553	-1553	—	BACK	VERT	TOTAL
N	12-2-4	-1553	-1553	—	BACK	VERT	TOTAL
O	13-8-4	-1560	-1560	—	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.47")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.47")
CALCULATED VERT. DEFL.(TL) = L/ 954 (0.18")

CSI: TC=0.05/1.00 (C-D:1) , BC=0.68/1.00 (G-H:1) , WB=0.76/1.00 (B-H:1) , SSI=0.73/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 CCMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (B) (INPUT = 0.90)
JSI METAL= 0.51 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM B095-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285414	T42Z	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Wed Aug 23 09:35:36 2017 Page 1

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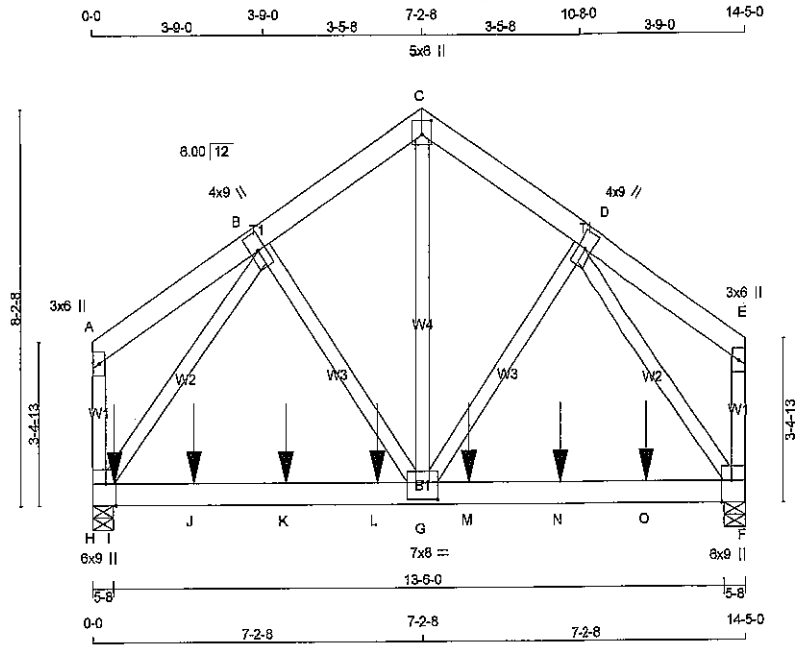

TOTAL WEIGHT = 3 X 99 = 297 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
H - F	2x6	DRY	1650F 1.5E	SPF
ALL WEBS	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS		
A- C 2	12	TOP
C- E 2	12	TOP
H- A 1	7	SIDE(176.5)
F- E 1	7	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS		
H- F 2	9	SIDE(621.9)
WEBS : (0.122"X3") SPIRAL NAILS		
2x4 1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	6.0		
B	TMWW+t	MT20	4.0	9.0		
C	TTW+p	MT20	5.0	6.0	3.50	2.50
D	TMWW+t	MT20	4.0	9.0		
E	TMV+p	MT20	3.0	6.0		
F	BMVW1+p	MT20	6.0	9.0	5.50	2.50
G	BMWWW-t	MT20	7.0	8.0	4.25	4.00
H	BMVW1+p	MT20	6.0	9.0	5.50	2.50

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1560.2 lbs FACTORED DOWN AT 5-12, 1552.9 lbs FACTORED DOWN AT 2-2-12, 1552.9 lbs FACTORED DOWN AT 4-2-12, 1552.9 lbs FACTORED DOWN AT 6-2-12, 1552.9 lbs FACTORED DOWN AT 8-2-12, AND 1552.9 lbs FACTORED DOWN AT 10-2-12, AND 1667.2 lbs FACTORED DOWN AT 12-2-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H	6980	0	6980	D	0	5-8	5-8
F	5630	0	5630	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	5684	3232 / 0	1244 / 0	0 / 0	0 / 0	1208 / 0	0 / 0
F	4572	2620 / 0	987 / 0	0 / 0	0 / 0	965 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-13 / 0	-84.3	-84.3	0.04 (1)	6.25	G- C	0 / 4551	0.24 (1)
B- C	-4399 / 0	-84.3	-84.3	0.05 (1)	6.25	G- D	0 / 1629	0.09 (1)
C- D	-4399 / 0	-84.3	-84.3	0.05 (1)	6.25	B- G	0 / 1629	0.09 (1)
D- E	-13 / 0	-84.3	-84.3	0.04 (1)	6.25	H- B	-4895 / 0	0.78 (1)
H- A	-167 / 0	0.0	0.0	0.01 (1)	7.81	D- F	-4895 / 0	0.78 (1)
F- E	-167 / 0	0.0	0.0	0.01 (1)	7.81			
H- I	0 / 2741	-28.0	-28.0	0.87 (1)	10.00			
I- J	0 / 2741	-28.0	-28.0	0.87 (1)	10.00			
J- K	0 / 2741	-28.0	-28.0	0.87 (1)	10.00			
K- L	0 / 2741	-28.0	-28.0	0.87 (1)	10.00			
L- G	0 / 2741	-28.0	-28.0	0.87 (1)	10.00			
G- M	0 / 2741	-28.0	-28.0	0.87 (1)	10.00			
M- N	0 / 2741	-28.0	-28.0	0.87 (1)	10.00			
N- O	0 / 2741	-28.0	-28.0	0.87 (1)	10.00			
O- F	0 / 2741	-28.0	-28.0	0.87 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
I	5-12	-1560	-1560	---	BACK	VERT	TOTAL
J	2-2-12	-1553	-1553	---	BACK	VERT	TOTAL
K	4-2-12	-1553	-1553	---	BACK	VERT	TOTAL
L	6-2-12	-1553	-1553	---	BACK	VERT	TOTAL
M	8-2-12	-1553	-1553	---	BACK	VERT	TOTAL
N	10-2-12	-1553	-1553	---	BACK	VERT	TOTAL
O	12-2-12	-1667	-1667	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.48")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.48")
CALCULATED VERT. DEFL.(TL) = L/ 748 (0.23")

CSI: TC=0.05 (C-D:1) , BC=0.87 (G-H:1) , WB=0.78 (D-F:1) , SSI=0.74 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (B) (INPUT = 0.90)
JSI METAL= 0.52 (D) (INPUT = 1.00)

DWG NO. TAM 4185J-17
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285414	T43	7	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Wed Aug 23 09:35:36 2017 Page 1
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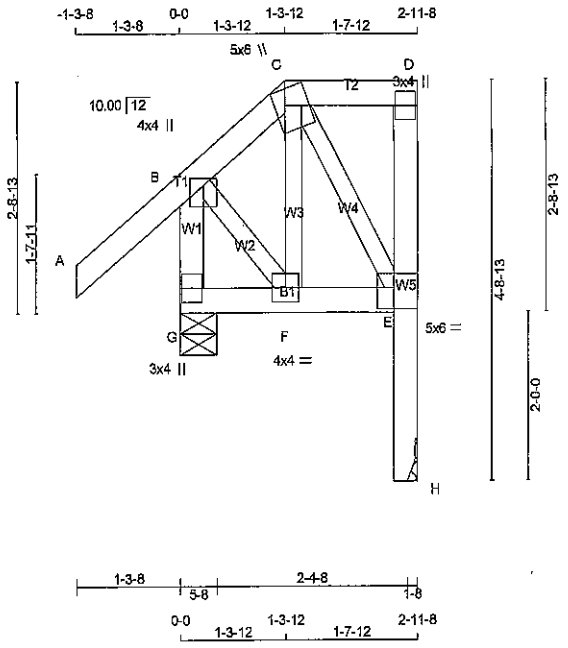


TOTAL WEIGHT = 7 X 21 = 144 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
H - D	2x4	DRY	No.2	SPF
G - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMV+p	MT20	3.0	4.0		
E	BVMW-I	MT20	5.0	6.0	3.00	2.50
F	BMWW-t	MT20	4.0	4.0		
G	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT	REQRD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG IN-SX	BRG IN-SX
H	139	0	139	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
G	310	0	310	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	118	59 / -17	31 / 0	0 / 0	0 / 0	28 / 0	0 / 0
G	234	163 / 0	31 / 0	0 / 0	0 / 0	40 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

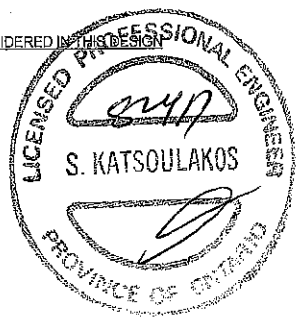
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 37	-84.3	-84.3	0.12 (1)	10.00	F- C	0 / 61	0.01 (3)	
B- C	-83 / 0	-84.3	-84.3	0.11 (1)	6.25	C- E	-62 / 11	0.01 (2)	
C- D	0 / 0	-84.3	-84.3	0.04 (1)	10.00	B- F	-11 / 45	0.01 (1)	
H- E	-139 / 0	0.0	0.0	0.02 (1)	7.81				
E- D	-69 / 0	0.0	0.0	0.01 (1)	7.81				
G- B	-292 / 0	0.0	0.0	0.03 (1)	7.81				
G- F	0 / 0	-28.0	-28.0	0.01 (3)	10.00				
F- E	-6 / 35	-28.0	-28.0	0.02 (2)	10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN



DWG NO. TAM 4/08-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.12 (A-B:1) , BC=0.02 (E-F:2) , WB=0.01 (C-F:3) , SSI=0.07 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285414	T44	21	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Wed Aug 23 09:35:37 2017 Page 1

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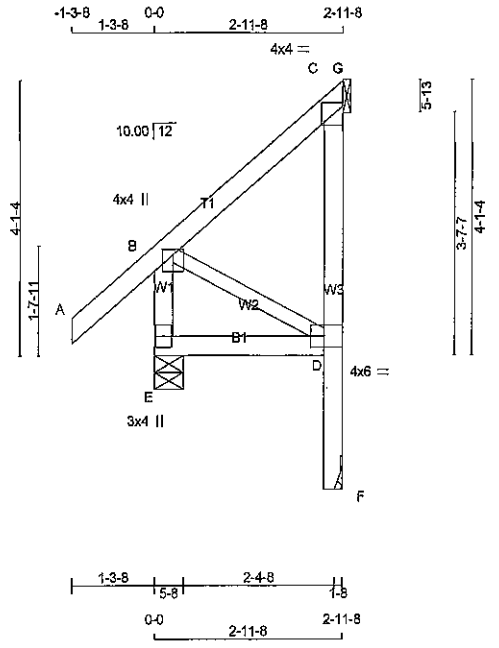

TOTAL WEIGHT = 21 X 20 = 414 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - G	2x4	DRY	No.2	SPF
F - C	2x4	DRY	No.2	SPF
E - B	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMV1-p	MT20	4.0	4.0		Edge
D	BVMW-I	MT20	4.0	6.0	2.00	2.50
E	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
C	137	0	137	0	0	1-8	1-8
F	24	0	46	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
E	277	0	277	0	0	5-8	5-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	102	74 / 0	12 / 0	0 / 0	0 / 0	16 / 0	0 / 0
F	32	0 / -1	20 / 0	0 / 0	0 / 0	13 / 0	0 / 0
E	211	143 / 0	31 / 0	0 / 0	0 / 0	37 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 37	-84.3	-84.3	0.12 (1)	10.00	B- D	-1 / 0	0.00 (2)
B- C	0 / 1	-84.3	-84.3	0.12 (1)	10.00			
C- G	0 / 0	0.0	0.0	0.00 (1)	10.00			
F- D	-46 / 0	0.0	0.0	0.01 (3)	7.81			
D- C	0 / 27	0.0	0.0	0.00 (2)	10.00			
E- B	-235 / 0	0.0	0.0	0.03 (1)	7.81			
E- D	0 / 0	-28.0	-28.0	0.07 (3)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DWG NO. TAM 41859-17

STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.12 (A-B:1) , BC=0.07 (D-E:3) , WB=0.00 (B-D:2) , SSI=0.07 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285414	T45	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Wed Aug 23 09:35:37 2017 Page 1

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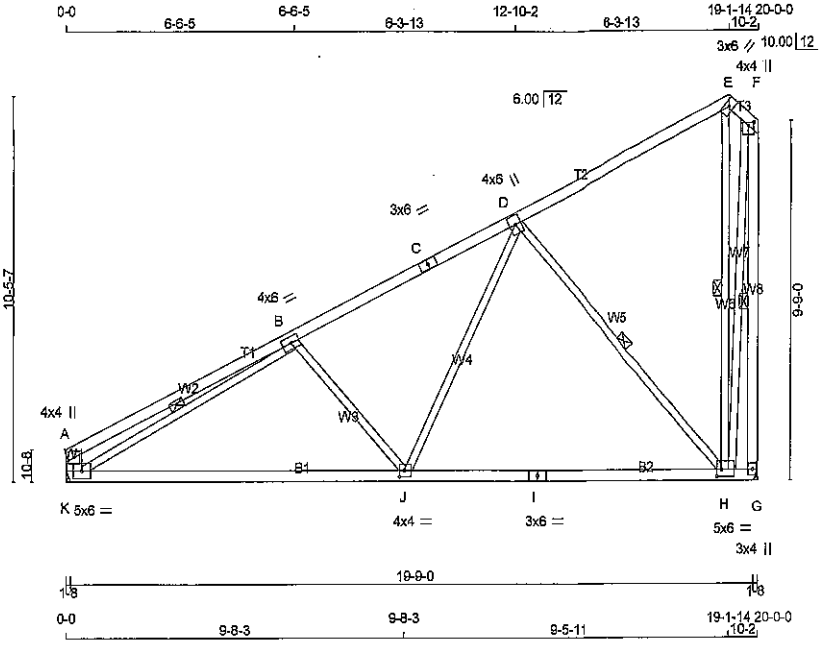


Scale = 1:62.8

TOTAL WEIGHT = 2 X 99 = 198 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - A	2x6	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	4.0	4.0		
B	TMWW-t	MT20	4.0	6.0		
C	TS-t	MT20	3.0	8.0		
D	TMWW+t	MT20	4.0	6.0		
E	TTW+h	MT20	3.0	6.0		
F	TMVW+p	MT20	4.0	4.0	1.00	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	6.0	2.25	1.50
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0	2.00	1.50
K	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1123	0	1123	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
K	1123	0	1123	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	922	512 / 0	210 / 0	0 / 0	0 / 0	200 / 0	0 / 0
K	922	512 / 0	210 / 0	0 / 0	0 / 0	200 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.39 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-H, E-H, F-G, B-K.

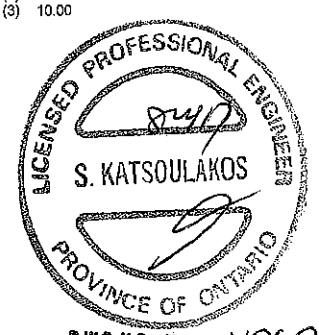
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 23	-84.3	-84.3	0.45 (1)	10.00
B-C	-1192 / 0	-84.3	-84.3	0.39 (1)	5.39
C-D	-1192 / 0	-84.3	-84.3	0.39 (1)	5.39
D-E	-145 / 0	-84.3	-84.3	0.39 (1)	6.25
E-F	-143 / 0	-84.3	-84.3	0.01 (1)	6.25
G-F	-1213 / 0	0.0	0.0	0.58 (1)	5.82
K-A	-219 / 0	0.0	0.0	0.01 (1)	7.81
K-J	0 / 1353	-28.0	-28.0	0.82 (2)	10.00
J-I	0 / 780	-28.0	-28.0	0.76 (2)	10.00
I-H	0 / 780	-28.0	-28.0	0.76 (2)	10.00
H-G	0 / 0	-28.0	-28.0	0.31 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B-J	-436 / 3	0.19 (1)
J-D	0 / 712	0.16 (2)
D-H	-1011 / 0	0.63 (1)
H-E	-102 / 75	0.08 (1)
H-F	0 / 1090	0.25 (1)
K-B	-1570 / 0	0.56 (1)



DWG NO. TAM 4/0617

STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/ 779 (0.31")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/ 468 (0.51")

CSI: TC=0.58 (F-G:1) , BC=0.82 (J-K:2) , WB=0.63 (D-H:1) , SSI=0.23 (J-K:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.44 (I) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285414	T45C	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Wed Aug 23 09:35:37 2017 Page 1
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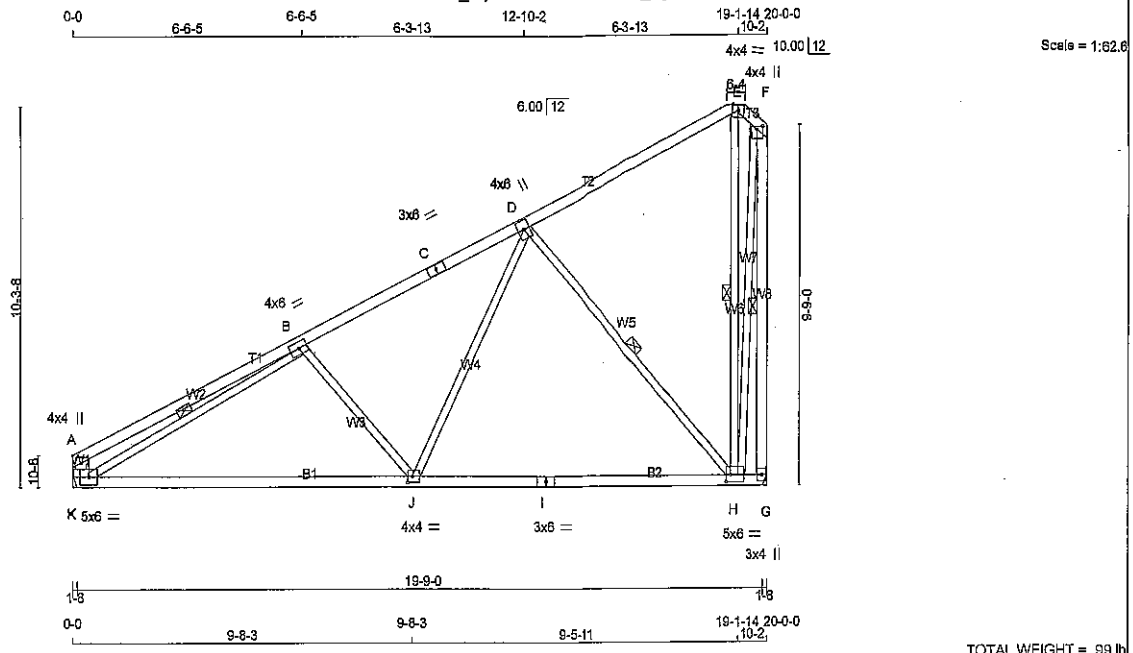


TOTAL WEIGHT = 99 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - A	2x6	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	4.0	4.0		
B	TMWW-t	MT20	4.0	6.0		
C	TS-t	MT20	3.0	6.0		
D	TMWW+t	MT20	4.0	6.0		
E	TTW-m	MT20	4.0	4.0	Edge	
F	TMVW+p	MT20	4.0	4.0	1.00	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	6.0	2.25	1.50
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0	2.00	1.50
K	BMVW1-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
G	1123	0	1123	0	0	IN-SX	IN-SX
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	
K	1123	0	1123	0	0	HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	922	512 / 0	210 / 0	0 / 0	0 / 0	200 / 0	0 / 0
K	922	512 / 0	210 / 0	0 / 0	0 / 0	200 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.39 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-H, E-H, F-G, B-K.

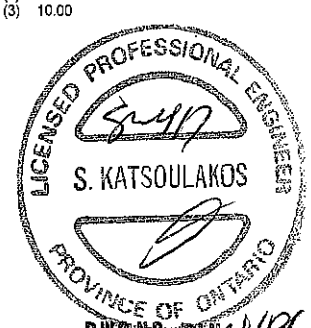
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 23	-84.3	-84.3	0.45 (1)	10.00
B- C	-1192 / 0	-84.3	-84.3	0.39 (1)	5.39
C- D	-1192 / 0	-84.3	-84.3	0.39 (1)	5.39
D- E	-145 / 0	-84.3	-84.3	0.39 (1)	6.25
E- F	-143 / 0	-84.3	-84.3	0.01 (1)	6.25
G- F	-1213 / 0	0.0	0.0	0.58 (1)	5.82
K- A	-219 / 0	0.0	0.0	0.01 (1)	7.81
K- J	0 / 1353	-28.0	-28.0	0.82 (2)	10.00
J- I	0 / 780	-28.0	-28.0	0.76 (2)	10.00
I- H	0 / 780	-28.0	-28.0	0.76 (2)	10.00
H- G	0 / 0	-28.0	-28.0	0.31 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- J	-436 / 3	0.19 (1)
J- D	0 / 712	0.16 (2)
D- H	-1011 / 0	0.63 (1)
H- E	-102 / 75	0.08 (1)
H- F	0 / 1090	0.25 (1)
K- B	-1570 / 0	0.56 (1)



DWG NO. T4M4/06/-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/ 779 (0.31")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/ 468 (0.51")

CSI: TC=0.58 (F-G:1) , BC=0.82 (J-K:2) , WB=0.63 (D-H:1) , SSI=0.23 (J-K:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.44 (I) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285420	T46	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Dec 12 2017 MiTek Industries, Inc. Tue Feb 6 11:10:27 2018 Page 1

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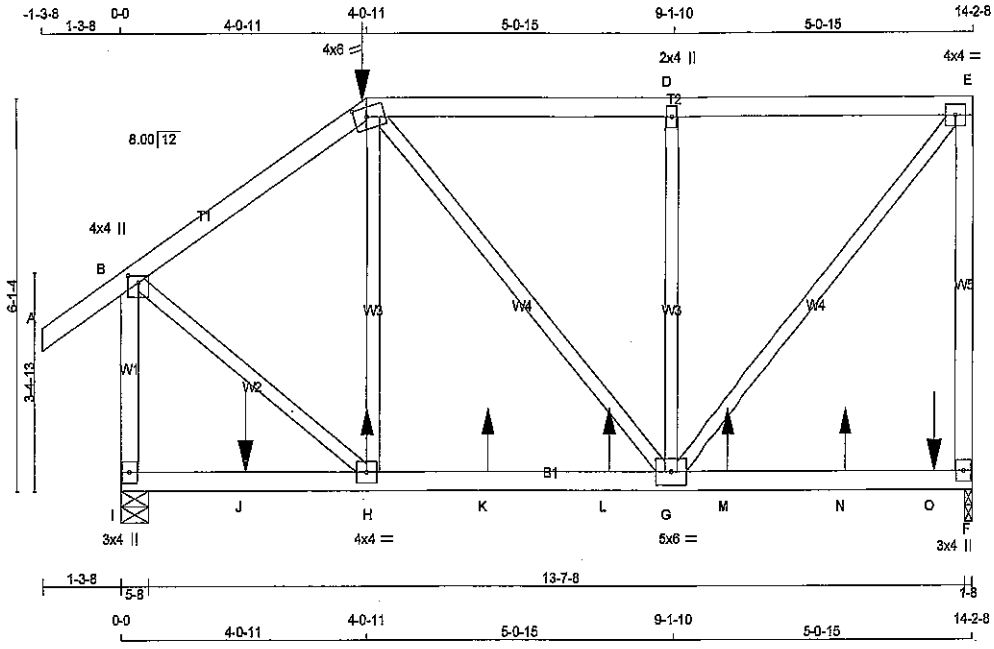

TOTAL WEIGHT = 70 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTWW-m	MT20	4.0	6.0	1.75	2.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	4.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	6.0		
H	BMWW-t	MT20	4.0	4.0		
I	BMV1+p	MT20	3.0	4.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 101.5 lbs FACTORED DOWN AT 4-0-11 ON TOP CHORD, AND 110.7 lbs FACTORED DOWN AT 2-0-12, 1.0 lbs FACTORED DOWN AND 1.0 lbs FACTORED UP AT 4-0-12, 1.0 lbs FACTORED DOWN AND 1.0 lbs FACTORED UP AT 6-0-12, 1.0 lbs FACTORED DOWN AND 1.0 lbs FACTORED UP AT 8-0-12, 1.0 lbs FACTORED DOWN AND 1.0 lbs FACTORED UP AT 10-0-12, AND 1.0 lbs FACTORED DOWN AND 1.0 lbs FACTORED UP AT 12-0-12, AND 5.8 lbs FACTORED DOWN AT 13-6-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	842	0	842	0	0	1-8	1-8
I	1076	0	1078	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	688	387 / 0	153 / 0	0 / 0	0 / 0	148 / 0	0 / 0
I	852	526 / 0	158 / 0	0 / 0	0 / 0	168 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 32	-84.3	-84.3	0.12 (1)	10.00	H- C	-157 / 87	0.09 (1)	
B- C	-597 / 0	-84.3	-84.3	0.27 (1)	6.25	C- G	0 / 73	0.02 (1)	
C- D	-541 / 0	-84.3	-84.3	0.40 (1)	6.25	G- D	-529 / 0	0.32 (1)	
D- E	-541 / 0	-84.3	-84.3	0.41 (1)	6.25	G- E	0 / 819	0.20 (1)	
F- E	-780 / 0	0.0	0.0	0.57 (1)	7.81	B- H	0 / 617	0.15 (1)	
I- B	-986 / 0	0.0	0.0	0.20 (1)	7.81				
I- J	0 / 0	-28.0	-28.0	0.19 (1)	10.00				
J- H	0 / 0	-28.0	-28.0	0.19 (1)	10.00				
H- K	0 / 493	-28.0	-28.0	0.24 (2)	10.00				
K- L	0 / 493	-28.0	-28.0	0.24 (2)	10.00				
L- G	0 / 493	-28.0	-28.0	0.24 (2)	10.00				
G- M	0 / 0	-28.0	-28.0	0.19 (3)	10.00				
M- N	0 / 0	-28.0	-28.0	0.19 (3)	10.00				
N- O	0 / 0	-28.0	-28.0	0.19 (3)	10.00				
O- F	0 / 0	-28.0	-28.0	0.19 (3)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	4-0-11	-10	-11	---	FRONT	VERT	DEAD
C	4-0-11	-92	-92	---	FRONT	VERT	SNOW
H	4-0-12	1	1	1	FRONT	VERT	TOTAL
J	2-0-12	-111	-111	---	FRONT	VERT	TOTAL
K	6-0-12	1	1	1	FRONT	VERT	TOTAL
L	6-0-12	1	1	1	FRONT	VERT	TOTAL
M	10-0-12	1	1	1	FRONT	VERT	TOTAL
N	12-0-12	1	1	1	FRONT	VERT	TOTAL
O	13-6-12	-2	-6	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.47")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.47")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.57/1.00 (E-F:1) , BC=0.24/1.00 (G-H:2) , WB=0.32/1.00 (D-G:1) , SSI=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (E) (INPUT = 0.90)
JSI METAL= 0.20 (E) (INPUT = 1.00)

DWG NO. TAM 8096-A8
STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285420	T47	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Dec 12 2017 MiTek Industries, Inc. Tue Feb 6 11:10:28 2018 Page 1
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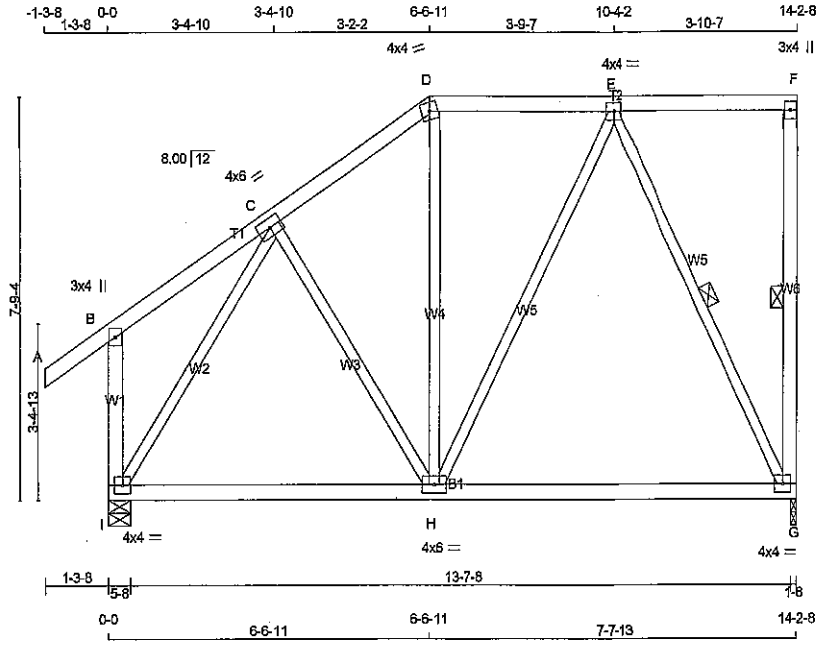

Scale = 1:44.6

TOTAL WEIGHT = 76 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BMWWW-t	MT20	4.0	6.0		
I	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
G	797	0	797	0	0	1-8	1-8
I	913	0	913	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	655	364 / 0	149 / 0	0 / 0	0 / 0	142 / 0	0 / 0
I	733	434 / 0	149 / 0	0 / 0	0 / 0	150 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, E-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	
B- C	0 / 18	-84.3	-84.3	0.15 (1)	10.00	
C- D	-498 / 0	-84.3	-84.3	0.15 (1)	6.25	
D- E	-399 / 0	-84.3	-84.3	0.21 (1)	6.25	
E- F	0 / 0	-84.3	-84.3	0.20 (1)	10.00	
G- F	-124 / 0	0.0	0.0	0.03 (1)	6.25	
I- B	-221 / 0	0.0	0.0	0.04 (1)	7.81	
I- H	0 / 388	-28.0	-28.0	0.46 (2)	10.00	
H- G	0 / 305	-28.0	-28.0	0.45 (2)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- H	0 / 109	0.02 (3)
H- D	0 / 132	0.03 (3)
H- E	0 / 261	0.06 (2)
E- G	-665 / 0	0.32 (1)
I- C	-736 / 0	0.48 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.47")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.47")
CALCULATED VERT. DEFL.(TL) = L/ 983 (0.17")

CSI: TC=0.21/1.00 (D-E:1) , BC=0.46/1.00 (H-I:2) , WB=0.48/1.00 (C-I:1) , SSI=0.18/1.00 (G-H:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (C) (INPUT = 0.90)
JSI METAL= 0.17 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM B097 -R
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285420	T48	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Dec 12 2017 MiTek Industries, Inc. Tue Feb 6 11:10:28 2018 Page 1
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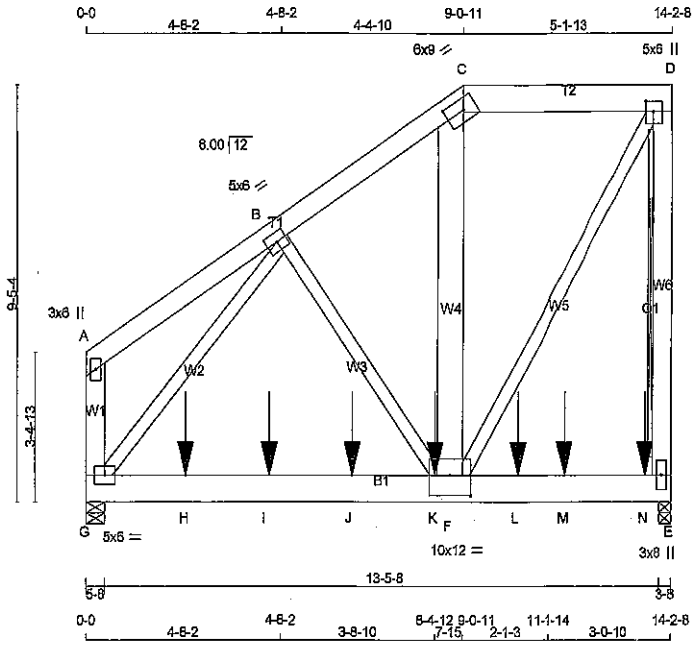

TOTAL WEIGHT = 3 X 131 = 392 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - D	2x6	DRY	No.2	SPF
E - D	2x6	DRY	No.2	SPF
G - A	2x6	DRY	No.2	SPF
G - E	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
F - C	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)		LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	TOP
D- E	2	12	TOP
G- A	2	12	TOP
C- D	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
G- E	2	9	SIDE(571.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	
F- C	2	6	SIDE(148.3)

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	6.0		
B	TMWW-t	MT20	5.0	6.0		
C	TTW-h	MT20	6.0	9.0	3.00	5.25
D	TMVW+p	MT20	5.0	6.0	2.75	2.25
E	BMV1+p	MT20	3.0	8.0		
F	BMWWW-t	MT20	10.0	12.0	5.50	6.00
G	BMVW1-t	MT20	5.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1667.2 lbs FACTORED DOWN AT 2-4-12, 1667.2 lbs FACTORED DOWN AT 4-4-12, 1552.9 lbs FACTORED DOWN AT 8-4-12, 2181.0 lbs FACTORED DOWN AT 8-4-12, 1094.5 lbs FACTORED DOWN AT 10-4-12, AND 1094.5 lbs FACTORED DOWN AT 11-6-12, AND 1099.5 lbs FACTORED DOWN AT 13-6-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	6324	0	6324	0	0	3-8	3-8
G	5628	0	5628	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	5131	2947 / 0	1103 / 0	0 / 0	0 / 0	1081 / 0	0 / 0
G	4556	2635 / 0	967 / 0	0 / 0	0 / 0	954 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x6 DRY SPF No.2 T-BRACE AT D-E

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-3 / 15	-84.3	-84.3	0.04 (1)	10.00	F- C	0 / 1765	0.06 (1)
B- C	-3917 / 0	-84.3	-84.3	0.05 (1)	6.25	B- F	0 / 1029	0.06 (1)
C- D	-3190 / 0	-84.3	-84.3	0.05 (1)	6.25	G- B	-4325 / 0	0.96 (1)
E- D	-5394 / 0	0.0	0.0	0.40 (1)	7.81	F- D	0 / 3074	0.33 (1)
G- A	-162 / 0	0.0	0.0	0.01 (1)	7.81			
G- H	0 / 2643	-28.0	-28.0	0.91 (1)	10.00			
H- I	0 / 2643	-28.0	-28.0	0.91 (1)	10.00			
I- J	0 / 2643	-28.0	-28.0	0.91 (1)	10.00			
J- K	0 / 2643	-28.0	-28.0	0.91 (1)	10.00			
K- F	0 / 2643	-28.0	-28.0	0.91 (1)	10.00			
F- L	0 / 0	-28.0	-28.0	0.82 (1)	10.00			
L- M	0 / 0	-28.0	-28.0	0.82 (1)	10.00			
M- N	0 / 0	-28.0	-28.0	0.82 (1)	10.00			
N- E	0 / 0	-28.0	-28.0	0.82 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
H	2-4-12	-1667	-1667	---	BACK	VERT	TOTAL
I	4-4-12	-1667	-1667	---	BACK	VERT	TOTAL
J	6-4-12	-1553	-1553	---	BACK	VERT	TOTAL
K	8-4-12	-2181	-2181	---	BACK	VERT	TOTAL
L	10-4-12	-1095	-1095	---	BACK	VERT	TOTAL
M	11-6-12	-1095	-1095	---	BACK	VERT	TOTAL
N	13-6-12	-1099	-1099	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.47")
CALCULATED VERT. DEFL.(LL) = L/ 905 (0.19")
ALLOWABLE DEFL.(TL)= L/360 (0.47")
CALCULATED VERT. DEFL.(TL) = L/ 566 (0.30")

CSI: TC=0.40/1.00 (D-E:1) , BC=0.91/1.00 (F-G:1) , WB=0.96/1.00 (B-G:1) , SSI=0.94/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SSI GRIP= 0.87 (D) (INPUT = 0.90)
SSI METAL= 0.95 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 8096-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285414	T49X	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Wed Aug 23 09:35:38 2017 Page 1
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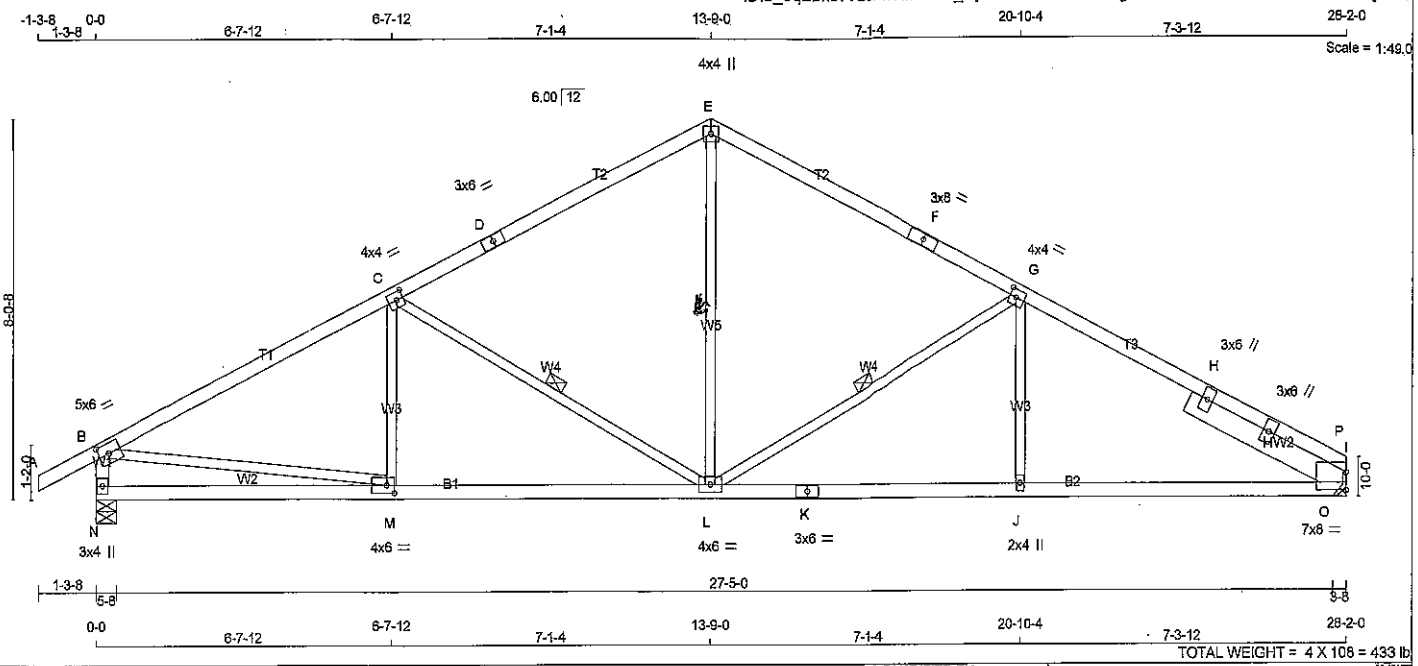

TOTAL WEIGHT = 4 X 108 = 433 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
REINFORCING MEMBERS				
HW2	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	6.0		
E	TTW+p	MT20	4.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
I	TMBMR1-I	MT20	7.0	8.0	4.50	
I	RT+t	MT20	3.0	6.0		
I	RT+t	MT20	3.0	6.0		
J	BMW+w	MT20	2.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	4.0	6.0		
M	BMWW-t	MT20	4.0	6.0	2.00	2.00
N	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT	REQRD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	BRG
N	1695	0	1695	0	0	IN-SX	IN-SX
I	1581	0	1581	0	0	5-8	5-8
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1376	790 / 0	296 / 0	0 / 0	0 / 0	290 / 0	0 / 0
I	1298	721 / 0	296 / 0	0 / 0	0 / 0	282 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.87 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-L, G-L. E-L

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 26	-84.3	-84.3	0.11	(1)	10.00	M- C	-59 / 241	0.05 (3)
B- C	-2272 / 0	-84.3	-84.3	0.64	(1)	3.87	C- L	-703 / 0	0.33 (1)
C- D	-1657 / 0	-84.3	-84.3	0.61	(1)	4.42	L- E	0 / 970	0.22 (1)
D- E	-1657 / 0	-84.3	-84.3	0.61	(1)	4.42	L- G	-801 / 0	0.37 (1)
E- F	-1648 / 0	-84.3	-84.3	0.62	(1)	4.41	J- G	0 / 334	0.08 (3)
F- G	-1648 / 0	-84.3	-84.3	0.62	(1)	4.41	B- M	0 / 2078	0.47 (1)
G- H	-1797 / 0	-84.3	-84.3	0.49	(1)	4.49	O- P	0 / 1218	0.00 (1)
H- P	-1797 / 0	-84.3	-84.3	0.49	(1)	4.49	O- H	-1284 / 0	0.15 (1)
P- I	-2191 / 0	-84.3	-84.3	0.07	(2)	4.58			
N- B	-1619 / 0	0.0	0.0	0.16	(1)	6.50			
N- M	0 / 0	-28.0	-28.0	0.31	(3)	10.00			
M- L	0 / 2061	-28.0	-28.0	0.54	(2)	10.00			
L- K	0 / 2145	-28.0	-28.0	0.53	(2)	10.00			
K- J	0 / 2145	-28.0	-28.0	0.53	(2)	10.00			
J- O	0 / 2145	-28.0	-28.0	0.55	(2)	10.00			
O- I	0 / 1128	-28.0	-28.0	0.45	(2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.94")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.94")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.64 (B-C:1) , BC=0.55 (J-O:2) , WB=0.47 (B-M:1) , SSI=0.39 (I-O:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	518	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (M) (INPUT = 0.90)
JSI METAL= 0.62 (K) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO
8/24/17

DWG NO. TAM 41865 17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285414	T50	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Wed Aug 23 09:35:39 2017 Page 1

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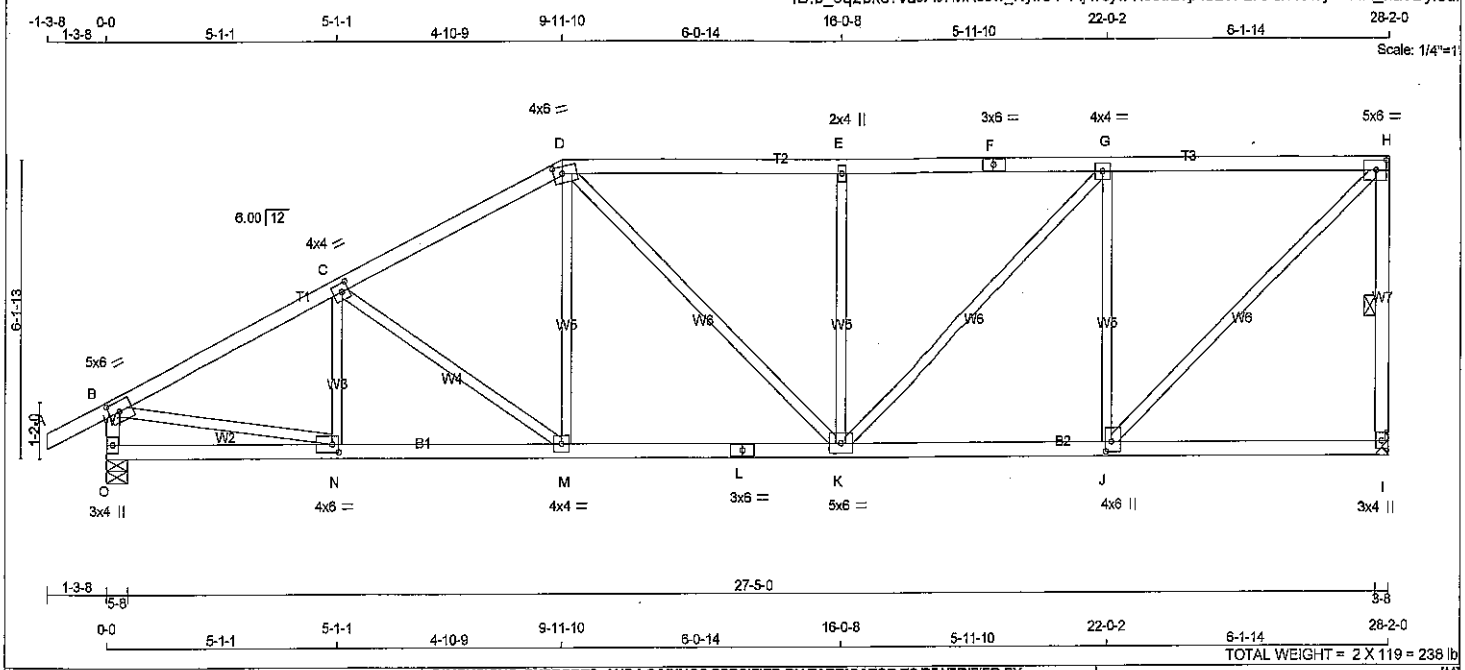


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	4.0	6.0	1.75	2.25
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TMVW-t	MT20	5.0	6.0	2.50	2.75
I	BMV1+p	MT20	3.0	4.0		
J	BMWW+t	MT20	4.0	6.0	2.50	1.50
K	BMWWW-t	MT20	5.0	6.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMWW-t	MT20	4.0	6.0	2.00	1.75
O	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
I	1581	0	1581	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	
O	1695	0	1695	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1298	721 / 0	296 / 0	0 / 0	0 / 0	282 / 0	0 / 0
O	1376	790 / 0	298 / 0	0 / 0	0 / 0	290 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.12 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF H-I.

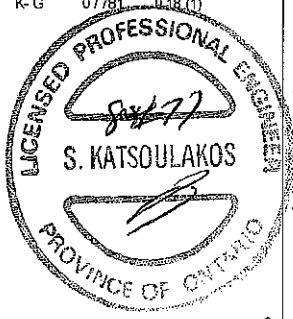
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 26	-84.3	-84.3	0.11 (1)	10.00
B- C	-2238 / 0	-84.3	-84.3	0.45 (1)	4.12
C- D	-1977 / 0	-84.3	-84.3	0.42 (1)	4.36
D- E	-1924 / 0	-84.3	-84.3	0.53 (1)	4.28
E- F	-1924 / 0	-84.3	-84.3	0.54 (1)	4.25
F- G	-1924 / 0	-84.3	-84.3	0.54 (1)	4.25
G- H	-1367 / 0	-84.3	-84.3	0.49 (1)	4.93
I- H	-1511 / 0	0.0	0.0	0.27 (1)	5.35
O- B	-1633 / 0	0.0	0.0	0.17 (1)	6.48
O- N	0 / 0	-28.0	-28.0	0.15 (3)	10.00
N- M	0 / 2020	-28.0	-28.0	0.42 (1)	10.00
M- L	0 / 1754	-28.0	-28.0	0.39 (2)	10.00
L- K	0 / 1754	-28.0	-28.0	0.39 (2)	10.00
K- J	0 / 1367	-28.0	-28.0	0.41 (2)	10.00
J- I	0 / 0	-28.0	-28.0	0.25 (3)	10.00

WEBS FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- C	-196 / 96	0.04 (1)
C- M	-330 / 0	0.21 (1)
M- D	0 / 395	0.09 (2)
B- N	0 / 2049	0.46 (1)
J- H	0 / 1667	0.42 (1)
D- K	0 / 236	0.05 (1)
J- G	-1109 / 0	0.66 (1)
K- E	-551 / 0	0.33 (1)
K- G	0 / 781	0.18 (1)



DWG NO. TAM4/066-17
STRUCTURAL COMPONENT ONLY

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.94")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.94")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.54 (E-G:1) , BC=0.42 (M-N:1) , WB=0.66 (G-J:1) , SSI=0.24 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (J) (INPUT = 0.90)
JSI METAL= 0.80 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285414	T51	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Wed Aug 23 09:35:39 2017 Page 1
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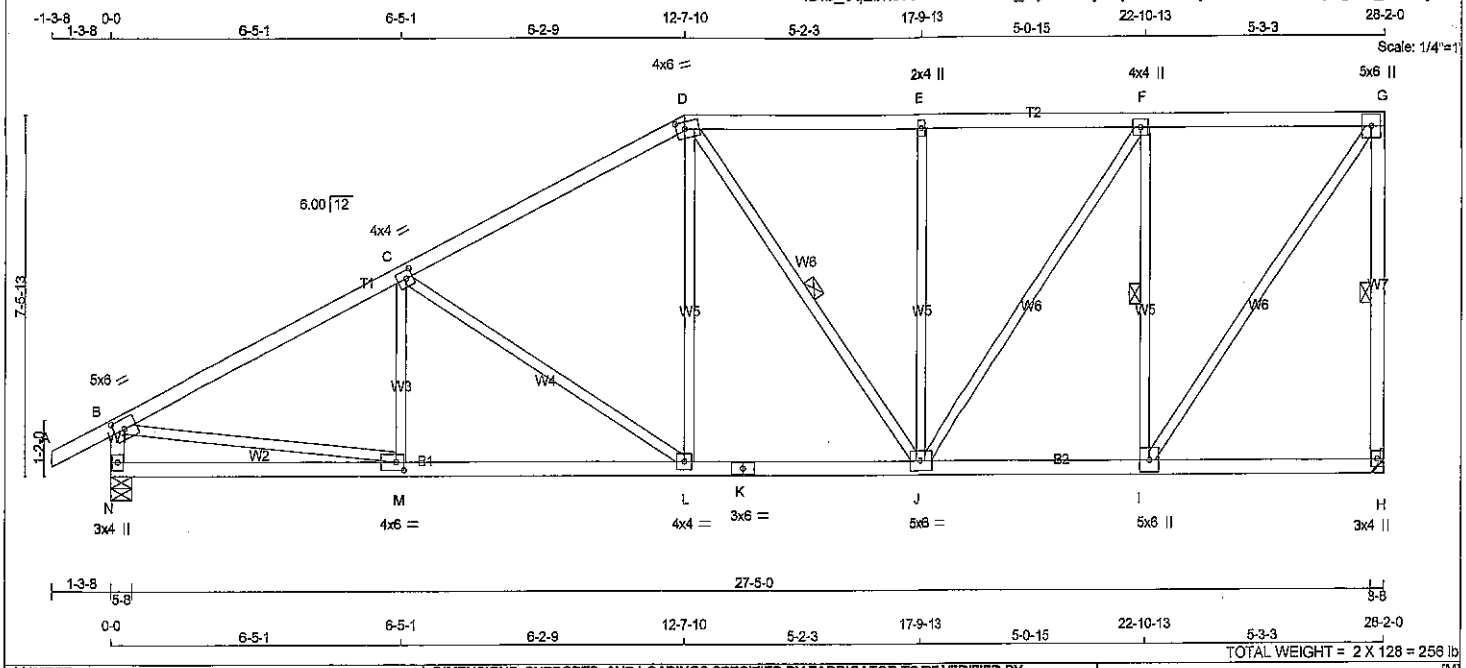


TOTAL WEIGHT = 2 X 128 = 256 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	4.0	6.0	1.75	2.25
E	TMW+w	MT20	2.0	4.0		
F	TMWW+t	MT20	4.0	4.0		
G	TMVW+p	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWW+t	MT20	5.0	6.0		
J	BMWWW-t	MT20	5.0	6.0		
K	BS-t	MT20	3.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMWW-t	MT20	4.0	6.0	2.00	2.00
N	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1581	0	1581	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	
N	1695	0	1695	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1298	721 / 0	296 / 0	0 / 0	0 / 0	282 / 0	0 / 0
N	1376	790 / 0	296 / 0	0 / 0	0 / 0	290 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.67 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

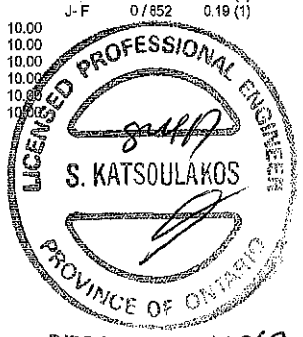
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-H, D-J, F-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 26	-84.3	-84.3	0.11 (1)	10.00
B- C	-2263 / 0	-84.3	-84.3	0.82 (1)	3.67
C- D	-1749 / 0	-84.3	-84.3	0.71 (1)	4.17
D- E	-1474 / 0	-84.3	-84.3	0.34 (1)	5.00
E- F	-1474 / 0	-84.3	-84.3	0.35 (1)	4.98
F- G	-982 / 0	-84.3	-84.3	0.33 (1)	5.64
H- G	-1522 / 0	0.0	0.0	0.38 (1)	5.33
N- B	-1621 / 0	0.0	0.0	0.16 (1)	6.50
N- M	0 / 0	-28.0	-28.0	0.28 (3)	10.00
M- L	0 / 2049	-28.0	-28.0	0.51 (2)	10.00
L- K	0 / 1545	-28.0	-28.0	0.35 (2)	10.00
K- J	0 / 1545	-28.0	-28.0	0.35 (2)	10.00
J- I	0 / 982	-28.0	-28.0	0.30 (2)	10.00
I- H	0 / 0	-28.0	-28.0	0.19 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- C	-76 / 220	0.05 (3)
C- L	-608 / 0	0.69 (1)
L- D	0 / 508	0.11 (2)
B- M	0 / 2087	0.47 (1)
I- G	0 / 1663	0.37 (1)
D- J	-121 / 0	0.06 (1)
I- F	-1175 / 0	0.41 (1)
J- E	-469 / 0	0.47 (1)
J- F	0 / 852	0.19 (1)



DWG NO. TAM 4067-17
STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.94")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.94")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.82 (B-C:1) , BC=0.51 (L-M:2) , WB=0.69 (C-L:1) , SSI=0.23 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (M) (INPUT = 0.90)
JSI METAL= 0.61 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285414	T52A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Wed Aug 23 09:35:39 2017 Page 1

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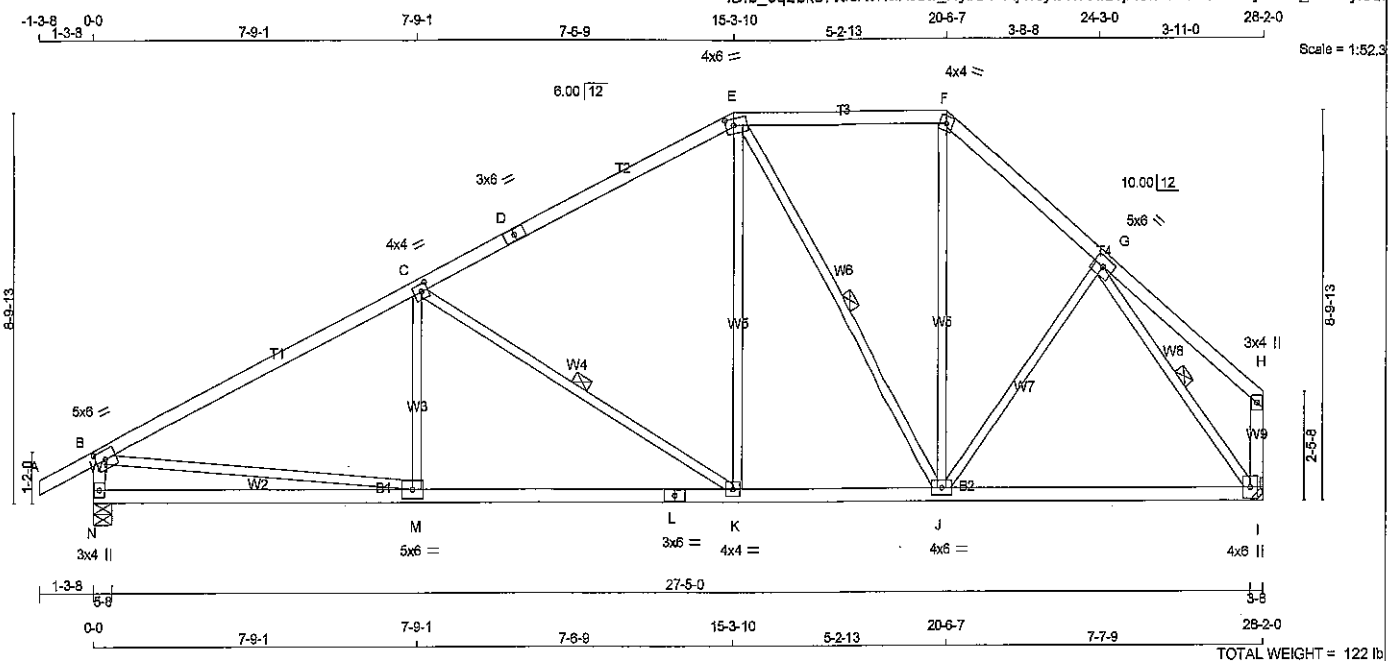

TOTAL WEIGHT = 122 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	4.0	6.0	1.75	2.25
F	TTW-m	MT20	4.0	4.0		
G	TMWW-t	MT20	5.0	6.0		
H	TMV+p	MT20	3.0	4.0		
I	BMVW1+p	MT20	4.0	6.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	5.0	6.0		
N	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
N	1695	0	1695	0	0	5-8	5-8
I	1581	0	1581	0	0	HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1376	790 / 0	296 / 0	0 / 0	0 / 0	290 / 0	0 / 0
I	1298	721 / 0	296 / 0	0 / 0	0 / 0	282 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.54 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-K, E-J, G-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 26	-84.3	-84.3	0.11 (1)	10.00	M- C	0 / 331	0.07 (3)
B- C	-2244 / 0	-84.3	-84.3	0.86 (1)	3.54	C- K	-869 / 0	0.49 (1)
C- D	-1488 / 0	-84.3	-84.3	0.72 (1)	4.36	K- E	0 / 608	0.14 (1)
D- E	-1488 / 0	-84.3	-84.3	0.72 (1)	4.36	E- J	-483 / 0	0.34 (1)
E- F	-1057 / 0	-84.3	-84.3	0.31 (1)	5.74	J- F	0 / 531	0.12 (1)
F- G	-1394 / 0	-84.3	-84.3	0.17 (1)	5.36	J- G	0 / 175	0.04 (2)
G- H	0 / 23	-84.3	-84.3	0.19 (1)	10.00	B- M	0 / 2051	0.48 (1)
N- B	-1606 / 0	0.0	0.0	0.16 (1)	6.52	G- I	-1682 / 0	0.48 (1)
I- H	-124 / 0	0.0	0.0	0.02 (1)	7.81			
N- M	0 / 0	-28.0	-28.0	0.44 (3)	10.00			
M- L	0 / 2038	-28.0	-28.0	0.67 (2)	10.00			
L- K	0 / 2038	-28.0	-28.0	0.67 (2)	10.00			
K- J	0 / 1306	-28.0	-28.0	0.50 (2)	10.00			
J- I	0 / 980	-28.0	-28.0	0.46 (2)	10.00			

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 4/06/17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.94")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.94")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.26")

CSI: TC=0.86 (B-C:1) , BC=0.67 (K-M:2) , WB=0.49 (C-K:1) , SSI=0.28 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1867	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (G) (INPUT = 0.90)
JSI METAL= 0.61 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285414	T53A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Wed Aug 23 09:35:40 2017 Page 1

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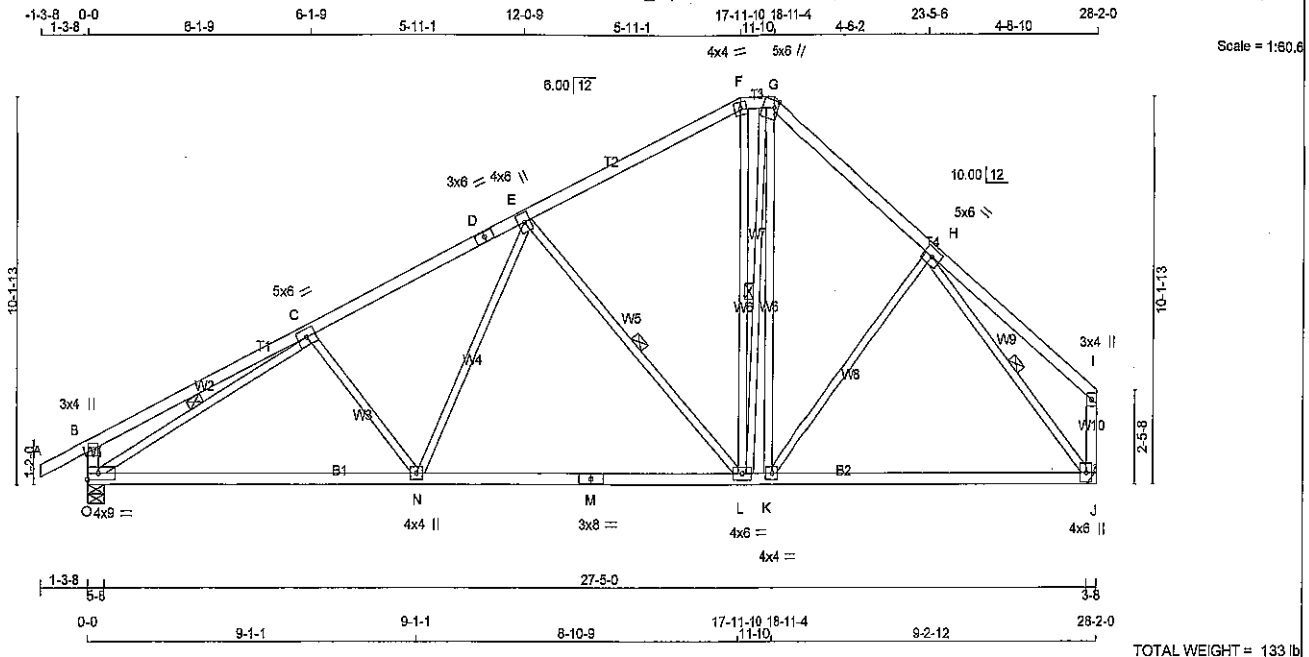

TOTAL WEIGHT = 133 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TS-t	MT20	3.0	6.0		
E	TMWW+t	MT20	4.0	6.0		
F	TTW-m	MT20	4.0	4.0		
G	TTWW+m	MT20	5.0	6.0	2.25	1.50
H	TMWW-t	MT20	5.0	6.0		
I	TMV+p	MT20	3.0	4.0		
J	BMVW1+p	MT20	4.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMWWW-t	MT20	4.0	6.0		
M	BS-t	MT20	3.0	8.0		
N	BMWW+t	MT20	4.0	4.0		
O	BMVW1-t	MT20	4.0	9.0		Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
O	1695	0	1695	0	0	5-8	5-8
J	1581	0	1581	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1376	790 / 0	296 / 0	0 / 0	0 / 0	290 / 0	0 / 0
J	1298	721 / 0	296 / 0	0 / 0	0 / 0	282 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.38 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-L, G-L, C-O, H-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 26	-84.3	-84.3	0.11 (1)	10.00	C- N	-248 / 79	0.10 (1)
B- C	0 / 21	-84.3	-84.3	0.39 (1)	10.00	N- E	0 / 564	0.13 (2)
C- D	-2039 / 0	-84.3	-84.3	0.40 (1)	4.38	E- L	-866 / 0	0.51 (1)
D- E	-2039 / 0	-84.3	-84.3	0.40 (1)	4.38	L- F	0 / 309	0.07 (2)
E- F	-1197 / 0	-84.3	-84.3	0.37 (1)	5.41	L- G	-30 / 312	0.07 (1)
F- G	-1053 / 0	-84.3	-84.3	0.02 (1)	6.12	K- G	0 / 486	0.11 (2)
G- H	-1363 / 0	-84.3	-84.3	0.25 (1)	5.30	K- H	-7 / 133	0.03 (3)
H- I	0 / 29	-84.3	-84.3	0.29 (1)	10.00	O- C	-2353 / 0	0.80 (1)
O- B	-321 / 0	0.0	0.0	0.03 (1)	7.81	H- J	-1681 / 0	0.63 (1)
J- I	-147 / 0	0.0	0.0	0.02 (1)	7.81			
O- N	0 / 1980	-28.0	-28.0	0.80 (2)	10.00			
N- M	0 / 1621	-28.0	-28.0	0.77 (2)	10.00			
M- L	0 / 1621	-28.0	-28.0	0.77 (2)	10.00			
L- K	0 / 1026	-28.0	-28.0	0.75 (2)	10.00			
K- J	0 / 1027	-28.0	-28.0	0.74 (2)	10.00			

DWG NO. TAM 4/09/17
STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.94")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.26")
ALLOWABLE DEFL.(TL)= L/360 (0.94")
CALCULATED VERT. DEFL.(TL) = L/ 773 (0.44")

CSI: TC=0.40 (C-E:1) , BC=0.80 (N-O:2) , WB=0.80 (C-O:1) , SSI=0.23 (K-L:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (O) (INPUT = 0.90)
JSI METAL= 0.65 (M) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285414	T54A	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Wed Aug 23 09:35:40 2017 Page 1

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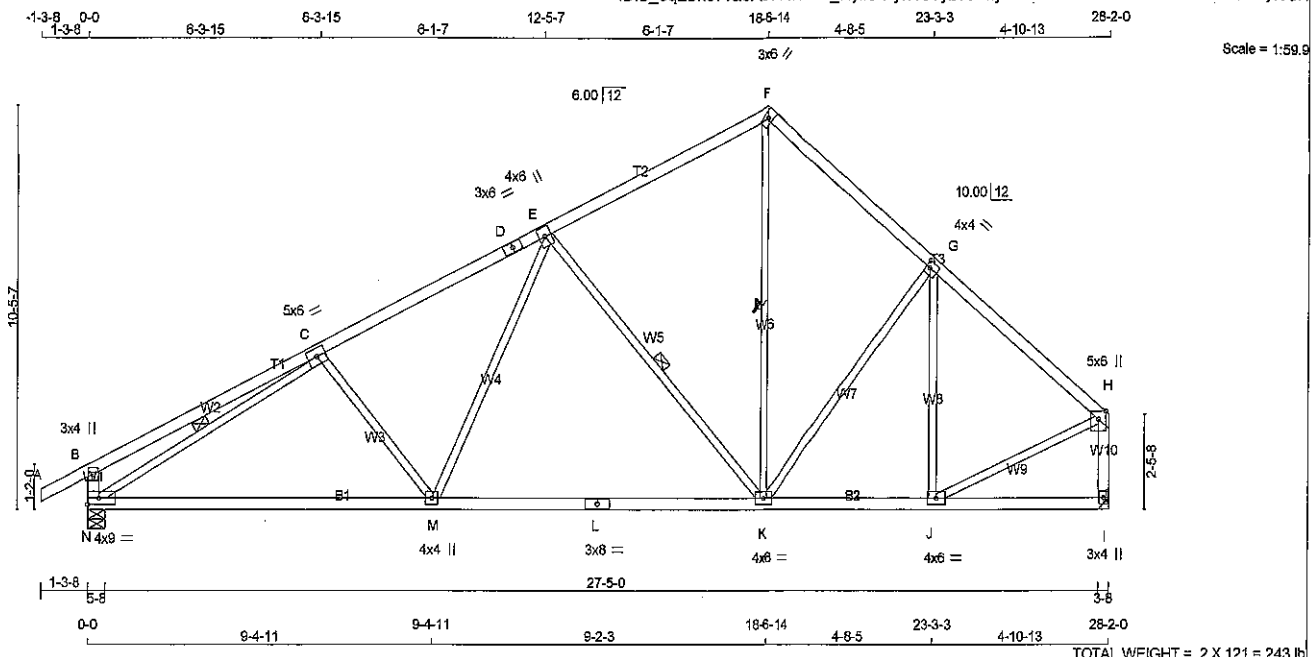

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TS-t	MT20	3.0	6.0		
E	TMWW+t	MT20	4.0	6.0		
F	TTW+h	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.25
H	TMVW+p	MT20	5.0	6.0	Edge	
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW+t	MT20	4.0	4.0		
N	BMVW1-t	MT20	4.0	9.0		Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT		
N	1695	0	1695	0	0	IN-SX	IN-SX
I	1581	0	1581	0	0	5-8	5-8
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1376	790 / 0	296 / 0	0 / 0	0 / 0	290 / 0	0 / 0
I	1298	721 / 0	296 / 0	0 / 0	0 / 0	282 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.37 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-K, C-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 26	-84.3	-84.3	0.11 (1)	10.00
B- C	0 / 22	-84.3	-84.3	0.42 (1)	10.00
C- D	-2019 / 0	-84.3	-84.3	0.43 (1)	4.37
D- E	-2019 / 0	-84.3	-84.3	0.43 (1)	4.37
E- F	-1148 / 0	-84.3	-84.3	0.39 (1)	5.46
F- G	-1331 / 0	-84.3	-84.3	0.27 (1)	5.32
G- H	-1369 / 0	-84.3	-84.3	0.28 (1)	5.26
N- B	-328 / 0	0.0	0.0	0.03 (1)	7.81
I- H	-1521 / 0	0.0	0.0	0.20 (1)	6.86
N- M	0 / 1979	-28.0	-28.0	0.65 (2)	10.00
M- L	0 / 1589	-28.0	-28.0	0.81 (2)	10.00
L- K	0 / 1589	-28.0	-28.0	0.81 (2)	10.00
K- J	0 / 1075	-28.0	-28.0	0.41 (2)	10.00
J- I	0 / 0	-28.0	-28.0	0.15 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- M	-274 / 73	0.12 (1)
M- E	0 / 599	0.13 (2)
E- K	-698 / 0	0.55 (1)
K- F	0 / 979	0.22 (1)
K- G	-127 / 0	0.16 (1)
J- G	-337 / 0	0.24 (1)
N- C	-2347 / 0	0.85 (1)
J- H	0 / 1171	0.28 (1)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 41879-17
STRUCTURAL COMPONENT ONLY

[M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.94")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.29")
ALLOWABLE DEFL.(TL)= L/360 (0.94")
CALCULATED VERT. DEFL.(TL) = L/ 694 (0.49")

CSI: TC=0.43 (C-E:1) , BC=0.65 (M-N:2) , WB=0.85 (C-N:1) , SSI=0.22 (M-N:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (M) (INPUT = 0.90)
JSI METAL= 0.67 (L) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285414	T55A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Wed Aug 23 09:35:41 2017 Page 1

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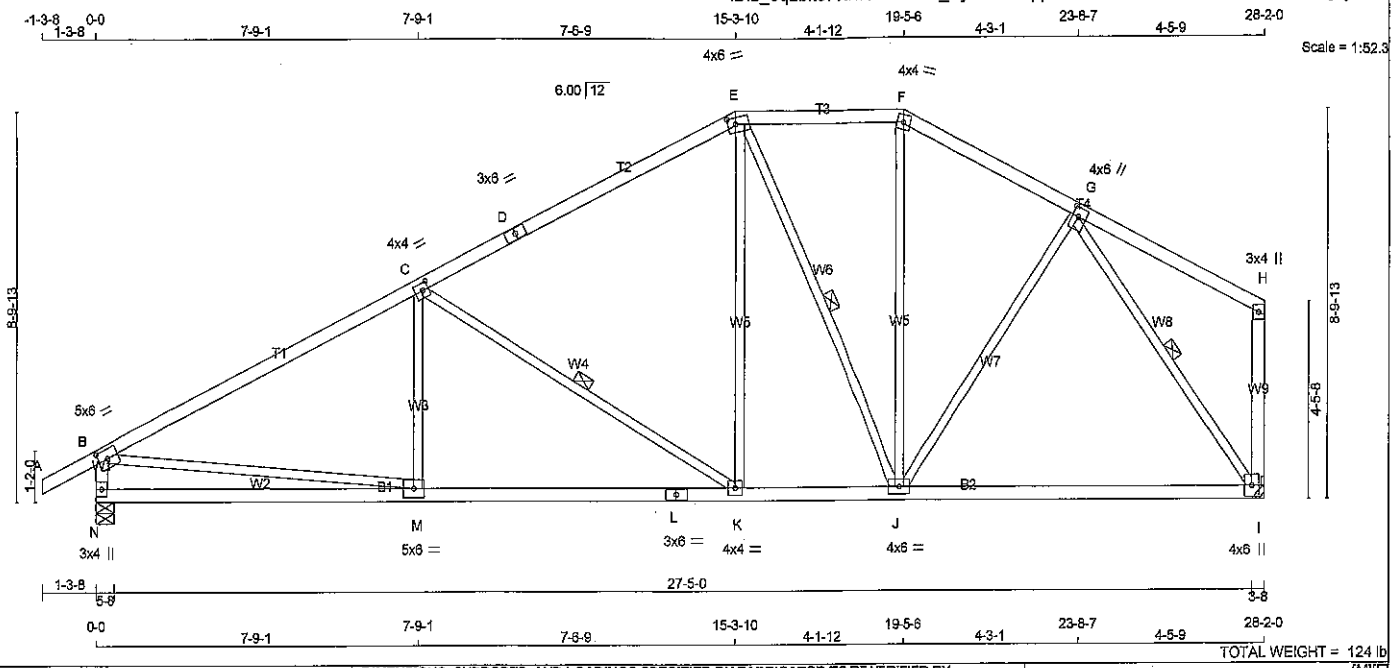


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW-t	MT20	5.0	6.0	2.50	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	4.0	6.0	1.75	2.25
F	TTW-m	MT20	4.0	4.0		
G	TMWW+t	MT20	4.0	6.0	2.50	1.75
H	TMV+p	MT20	3.0	4.0		
I	BMVW1+p	MT20	4.0	6.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	5.0	6.0		
N	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
N	1695	0	1695	0	0	5-8	5-8
I	1581	0	1581	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1376	790 / 0	296 / 0	0 / 0	0 / 0	290 / 0	0 / 0
I	1298	721 / 0	296 / 0	0 / 0	0 / 0	282 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.54 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

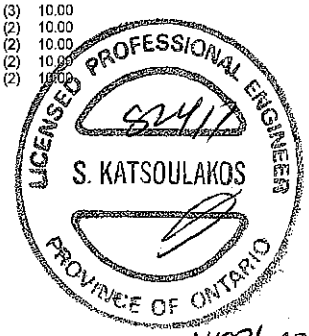
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-K, E-J, G-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 26	-84.3	-84.3	1	0.11 (1)	10.00	M- C	0 / 336	0.08 (3)
B- C	-2245 / 0	-84.3	-84.3	1	0.86 (1)	3.54	C- K	-873 / 0	0.49 (1)
C- D	-1486 / 0	-84.3	-84.3	1	0.72 (1)	4.36	K- E	0 / 571	0.13 (1)
D- E	-1486 / 0	-84.3	-84.3	1	0.72 (1)	4.36	E- J	-370 / 0	0.23 (1)
E- F	-1145 / 0	-84.3	-84.3	1	0.20 (1)	5.72	J- F	0 / 308	0.07 (2)
F- G	-1298 / 0	-84.3	-84.3	1	0.22 (1)	5.44	J- G	0 / 360	0.09 (2)
G- H	0 / 19	-84.3	-84.3	1	0.26 (1)	10.00	B- M	0 / 2051	0.46 (1)
N- B	-1809 / 0	0.0	0.0	1	0.16 (1)	6.52	G- I	-1627 / 0	0.65 (1)
I- H	-146 / 0	0.0	0.0	1	0.05 (1)	7.81			
N- M	0 / 0	-28.0	-28.0	1	0.45 (3)	10.00			
M- L	0 / 2039	-28.0	-28.0	1	0.68 (2)	10.00			
L- K	0 / 2039	-28.0	-28.0	1	0.68 (2)	10.00			
K- J	0 / 1304	-28.0	-28.0	1	0.62 (2)	10.00			
J- I	0 / 933	-28.0	-28.0	1	0.57 (2)	10.00			



DWG NO. TAM4/671-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.94")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.25")
ALLOWABLE DEFL.(TL)= L/360 (0.94")
CALCULATED VERT. DEFL.(TL) = L/ 827 (0.41")

CSI: TC=0.86 (B-C:1) , BC=0.68 (K-M:2) , WB=0.65 (G-I:1) , SSI=0.28 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (M) (INPUT = 0.90)
JSI METAL= 0.62 (L) (INPUT = 1.00)

(M)(F)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285414	T56A	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Wed Aug 23 09:35:41 2017 Page 1

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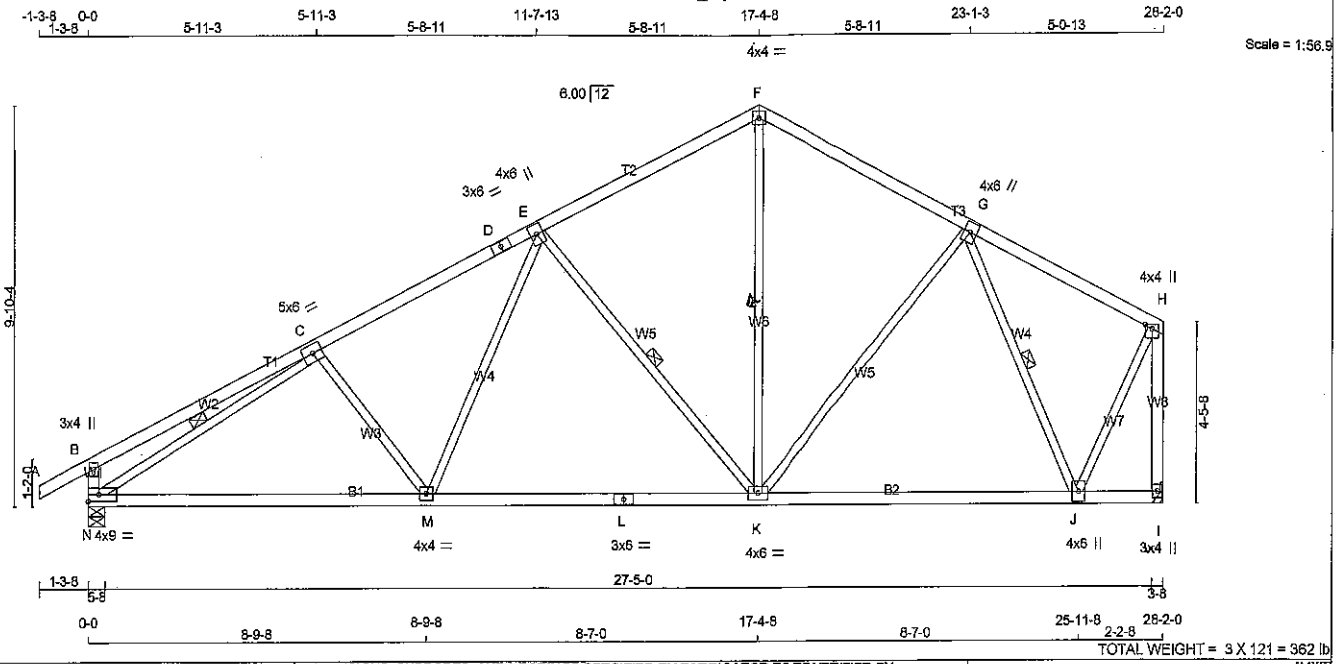


TOTAL WEIGHT = 3 X 121 = 362 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TS-t	MT20	3.0	6.0		
E	TMWW+t	MT20	4.0	6.0		
F	TTW-p	MT20	4.0	4.0		
G	TMWW+t	MT20	4.0	6.0		
H	TMVW+p	MT20	4.0	4.0	1.25	2.00
I	BMV1+p	MT20	3.0	4.0		
J	BMWW+t	MT20	4.0	6.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMVW1-t	MT20	4.0	9.0		Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
N	1695	0	1695	0	0	5-8	5-8
I	1581	0	1581	0	0	HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1376	790 / 0	296 / 0	0 / 0	0 / 0	290 / 0	0 / 0
I	1298	721 / 0	296 / 0	0 / 0	0 / 0	282 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.39 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

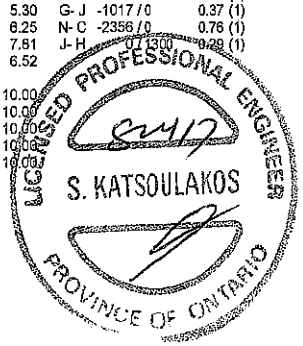
1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-K, G-J, C-N. F K

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI	MAX (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 26	-84.3	-84.3	0.11	(1)	10.00	C- M	-222 / 85	0.09 (1)
B- C	0 / 20	-84.3	-84.3	0.36	(1)	10.00	M- E	0 / 521	0.12 (2)
C- D	-2056 / 0	-84.3	-84.3	0.36	(1)	4.39	E- K	-818 / 0	0.44 (1)
D- E	-2056 / 0	-84.3	-84.3	0.38	(1)	4.39	K- F	0 / 738	0.17 (1)
E- F	-1274 / 0	-84.3	-84.3	0.35	(1)	5.32	K- G	0 / 187	0.04 (2)
F- G	-1275 / 0	-84.3	-84.3	0.35	(1)	5.30	G- J	-1017 / 0	0.37 (1)
G- H	-666 / 0	-84.3	-84.3	0.32	(1)	6.25	N- C	-2356 / 0	0.76 (1)
N- B	-315 / 0	0.0	0.0	0.03	(1)	7.81	J- H	0 / 1300	0.29 (1)
I- H	-1606 / 0	0.0	0.0	0.54	(1)	6.52			
N- M	0 / 1978	-28.0	-28.0	0.74	(2)	10.00			
M- L	0 / 1652	-28.0	-28.0	0.71	(2)	10.00			
L- K	0 / 1652	-28.0	-28.0	0.71	(2)	10.00			
K- J	0 / 1015	-28.0	-28.0	0.51	(2)	10.00			
J- I	0 / 0	-28.0	-28.0	0.32	(3)	10.00			



DWG NO. TAM41872-17

STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.94")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.25")
ALLOWABLE DEFL.(TL)= L/360 (0.94")
CALCULATED VERT. DEFL.(TL) = L/ 832 (0.41")

CSI: TC=0.54 (H-I:1) , BC=0.74 (M-N:2) , WB=0.76 (C-N:1) , SSI=0.20 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.57 (C) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285414	T99TCX	36	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Wed Aug 23 09:35:41 2017 Page 1
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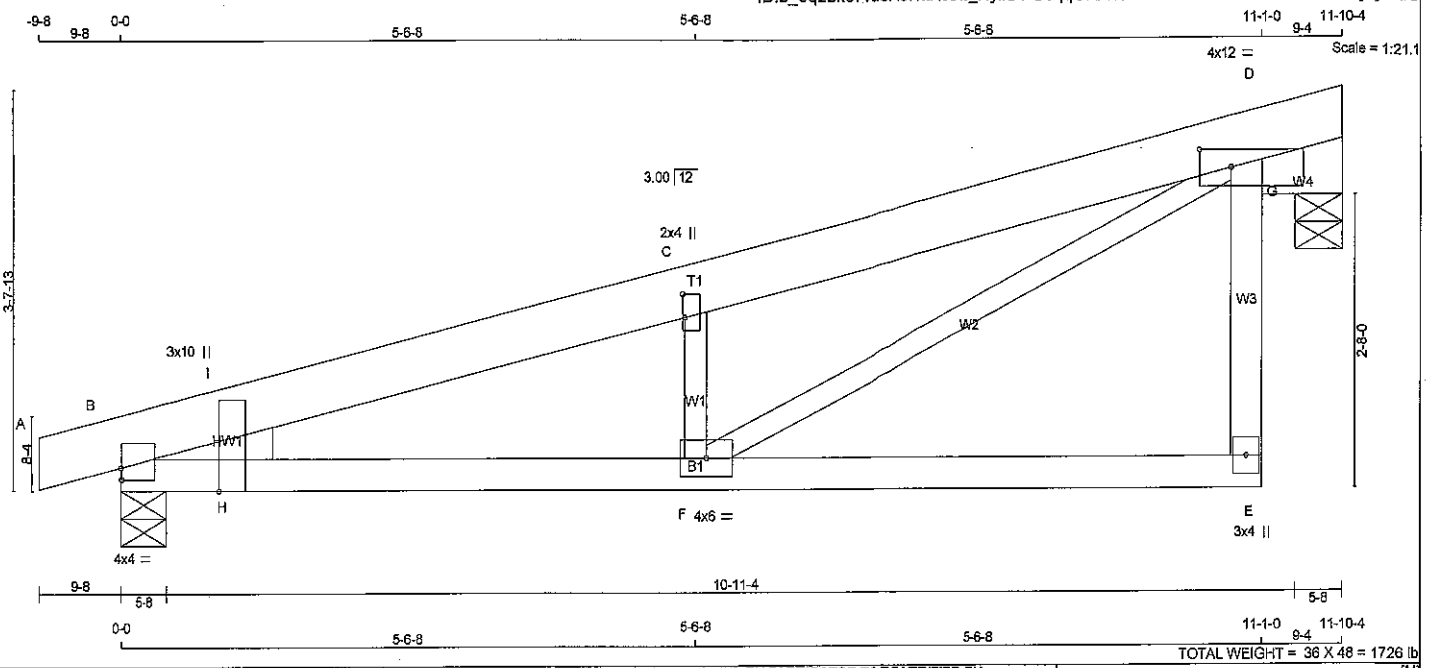


TOTAL WEIGHT = 36 X 48 = 1726 lb

(M)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	4.0	4.0	1.25	
B	WP+I	MT20	3.0	10.0	Edge	11.50
C	TMW+w	MT20	2.0	4.0	2.50	0.25
D	TMVWW1-p	MT20	4.0	12.0	2.00	3.75
E	BMV+p	MT20	3.0	4.0		
F	BMWW-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG	HEEL
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	WEDGE
J(D)	593	0	593	0	0	5-8	5-8	
B	722	0	722	0	0	5-8	5-8	2x4 L

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J(D)	487	270 / 0	111 / 0	0 / 0	0 / 0	106 / 0	0 / 0
B	583	340 / 0	122 / 0	0 / 0	0 / 0	121 / 0	0 / 0

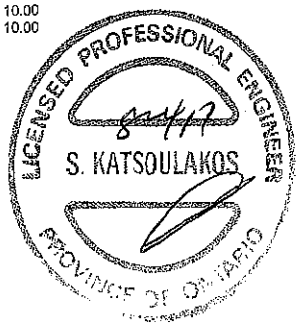
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J(D), B

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)		FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-4 / 0		-84.3	-84.3	0.02 (1)	10.00	F- C	-460 / 0	0.07 (1)
B- I	-1371 / 0		-84.3	-84.3	0.04 (1)	6.25	F- D	0 / 1271	0.29 (1)
I- C	-1265 / 0		-84.3	-84.3	0.10 (1)	6.25	H- I	0 / 145	0.00 (1)
C- D	-1259 / 0		-84.3	-84.3	0.19 (1)	6.25	D- J	-828 / 0	0.03 (1)
E- G	0 / 113		0.0	0.0	0.37 (1)	10.00	G- J	0 / 576	0.00 (1)
G- D	0 / 113		0.0	0.0	0.37 (1)	10.00			
B- H	0 / 1231		-28.0	-28.0	0.22 (1)	10.00			
H- F	0 / 1231		-28.0	-28.0	0.32 (2)	10.00			
F- E	0 / 104		-28.0	-28.0	0.19 (3)	10.00			



DWG NO. TAM41073 17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012', BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.37")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.37")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.37 (E-G:1) , BC=0.32 (F-H:2) , WB=0.29 (D-F:1) , SSI=0.16 (D-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (D) (INPUT = 0.90)
JSI METAL= 0.44 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285414	G99	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Wed Aug 23 09:35:19 2017 Page 1

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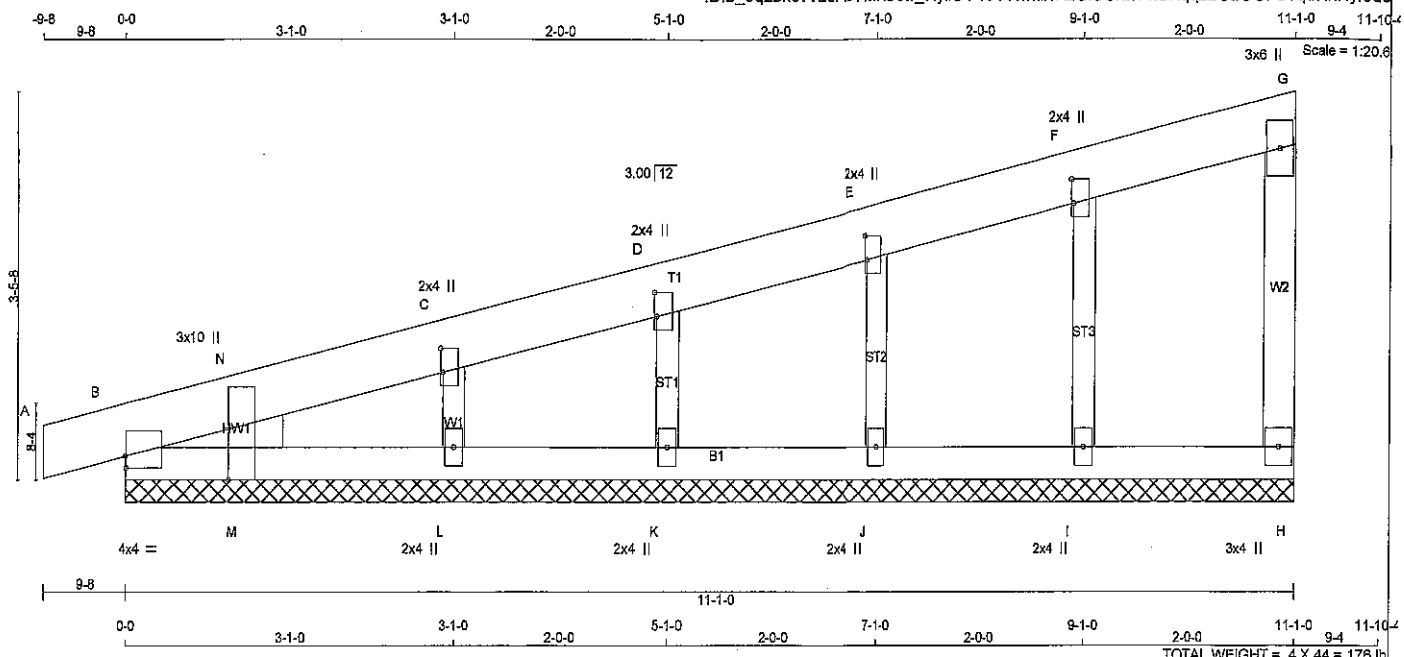


TOTAL WEIGHT = 4 X 44 = 176 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - G	2x6	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
B - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	4.0	4.0	1.25	
B	WP+I	MT20	3.0	10.0	Edge	11.50
C, D, E, F						
C	TMW+w	MT20	2.0	4.0	2.50	0.25
G	TMV+p	MT20	3.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I, J, K, L						
I	BMW1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

HEEL
WEDGE
2x4 L
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

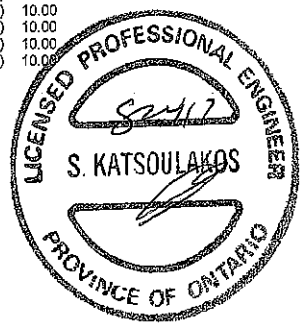
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-4 / 0	-84.3	-84.3	0.02 (1)	10.00
B- N	-57 / 0	-84.3	-84.3	0.04 (1)	6.25
N- C	-11 / 4	-84.3	-84.3	0.05 (1)	6.25
C- D	-20 / 0	-84.3	-84.3	0.05 (1)	6.25
D- E	-8 / 0	-84.3	-84.3	0.02 (1)	10.00
E- F	-4 / 0	-84.3	-84.3	0.02 (1)	10.00
F- G	-4 / 0	-84.3	-84.3	0.02 (1)	10.00
H- G	-72 / 0	0.0	0.0	0.01 (1)	7.81
B- M	0 / 27	-28.0	-28.0	0.03 (1)	10.00
M- L	0 / 27	-28.0	-28.0	0.04 (2)	10.00
L- K	0 / 14	-28.0	-28.0	0.04 (2)	10.00
K- J	0 / 9	-28.0	-28.0	0.02 (2)	10.00
J- I	0 / 4	-28.0	-28.0	0.03 (2)	10.00
I- H	0 / 0	-28.0	-28.0	0.03 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- F	-185 / 0	0.03 (1)
J- E	-171 / 0	0.03 (1)
K- D	-138 / 0	0.02 (1)
L- C	-256 / 0	0.04 (1)
M- N	0 / 63	0.00 (1)



DWG NO. TAM 4107-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.05 (C-D:1) , BC=0.04 (L-M:2) , WB=0.04 (C-L:1) , SSI=0.09 (C-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (C) (INPUT = 0.90)
JSI METAL= 0.05 (L) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285426	T112	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Thu Apr 19 13:26:56 2018 Page 1

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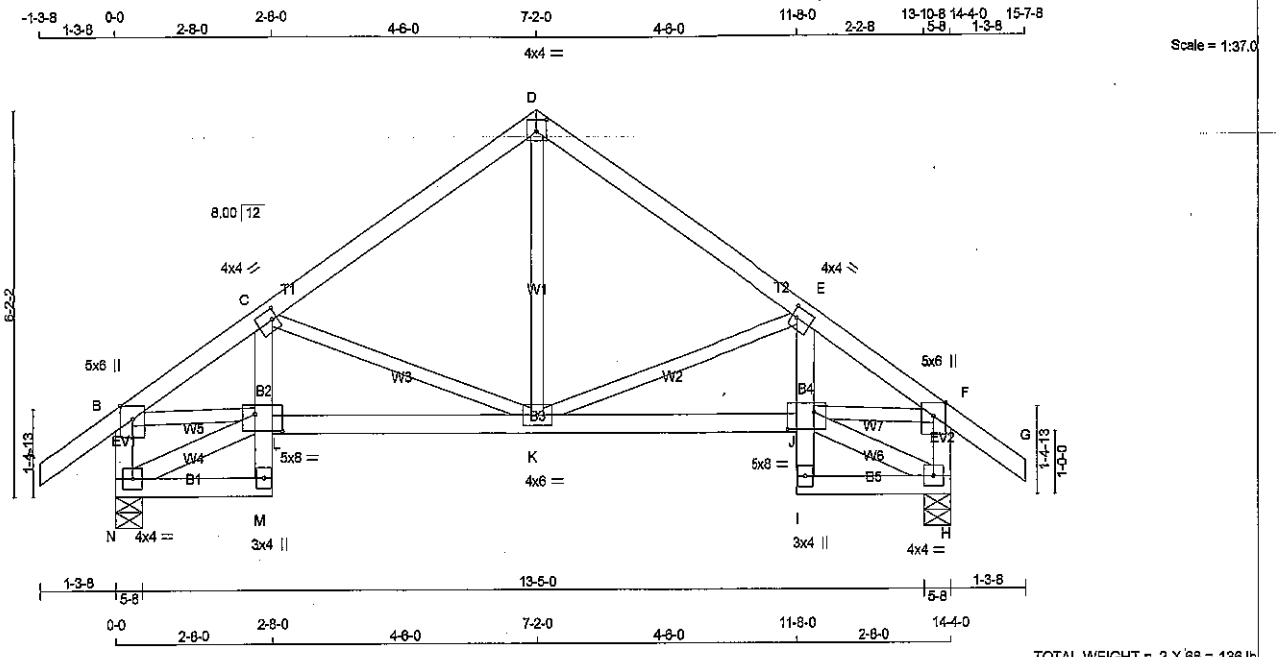

TOTAL WEIGHT = 2 X 68 = 136 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
M - C	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
I - E	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMVW-t	MT20	4.0	4.0	2.00	1.00
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMVW-t	MT20	4.0	4.0	2.00	1.00
F	TMVW+p	MT20	5.0	6.0	Edge	
H	BMVW1-t	MT20	4.0	4.0		
I	BMV+p	MT20	3.0	4.0		
J	BVMWW-I	MT20	5.0	8.0	3.25	5.50
K	BMWWW-t	MT20	4.0	6.0		
L	BVMWW-I	MT20	5.0	8.0	3.25	5.50
M	BMV+p	MT20	3.0	4.0		
N	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	920	0	920	0	0	5-8	5-8
H	920	0	920	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	739	437 / 0	150 / 0	0 / 0	0 / 0	152 / 0	0 / 0
H	739	437 / 0	150 / 0	0 / 0	0 / 0	152 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.60 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTOREO FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.3	-84.3	0.11 (1)		10.00
B- C	-1234 / 0	-84.3	-84.3	0.17 (1)		5.60
C- D	-748 / 0	-84.3	-84.3	0.22 (1)		6.25
D- E	-748 / 0	-84.3	-84.3	0.22 (1)		6.25
E- F	-1234 / 0	-84.3	-84.3	0.17 (1)		5.60
F- G	0 / 32	-84.3	-84.3	0.11 (1)		10.00
N- B	-869 / 0	0.0	0.0	0.09 (1)		7.81
H- F	-869 / 0	0.0	0.0	0.09 (1)		7.81
N- M	0 / 37	-28.0	-28.0	0.06 (2)		10.00
M- L	0 / 62	0.0	0.0	0.06 (1)		10.00
L- C	0 / 147	0.0	0.0	0.07 (1)		10.00
L- K	0 / 1076	-28.0	-28.0	0.30 (2)		10.00
K- J	0 / 1076	-28.0	-28.0	0.30 (2)		10.00
I- J	0 / 62	0.0	0.0	0.06 (1)		10.00
J- E	0 / 147	0.0	0.0	0.07 (1)		10.00
I- H	0 / 37	-28.0	-28.0	0.06 (2)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- D	0 / 462	0.11 (1)
K- E	-500 / 0	0.20 (1)
C- K	-500 / 0	0.20 (1)
N- L	-40 / 0	0.00 (1)
B- L	0 / 1053	0.24 (1)
J- H	-40 / 0	0.00 (1)
J- F	0 / 1053	0.24 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.48")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.48")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.22/1.00 (D-E:1) , BC=0.30/1.00 (J-K:2) , WB=0.24/1.00 (F-J:1) , SSI=0.15/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (L) (INPUT = 0.90)
JSI METAL= 0.24 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4,918
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM21189-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285415	T170S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Dec 12 2017 MiTek Industries, Inc. Tue Feb 6 16:55:10 2018 Page 1

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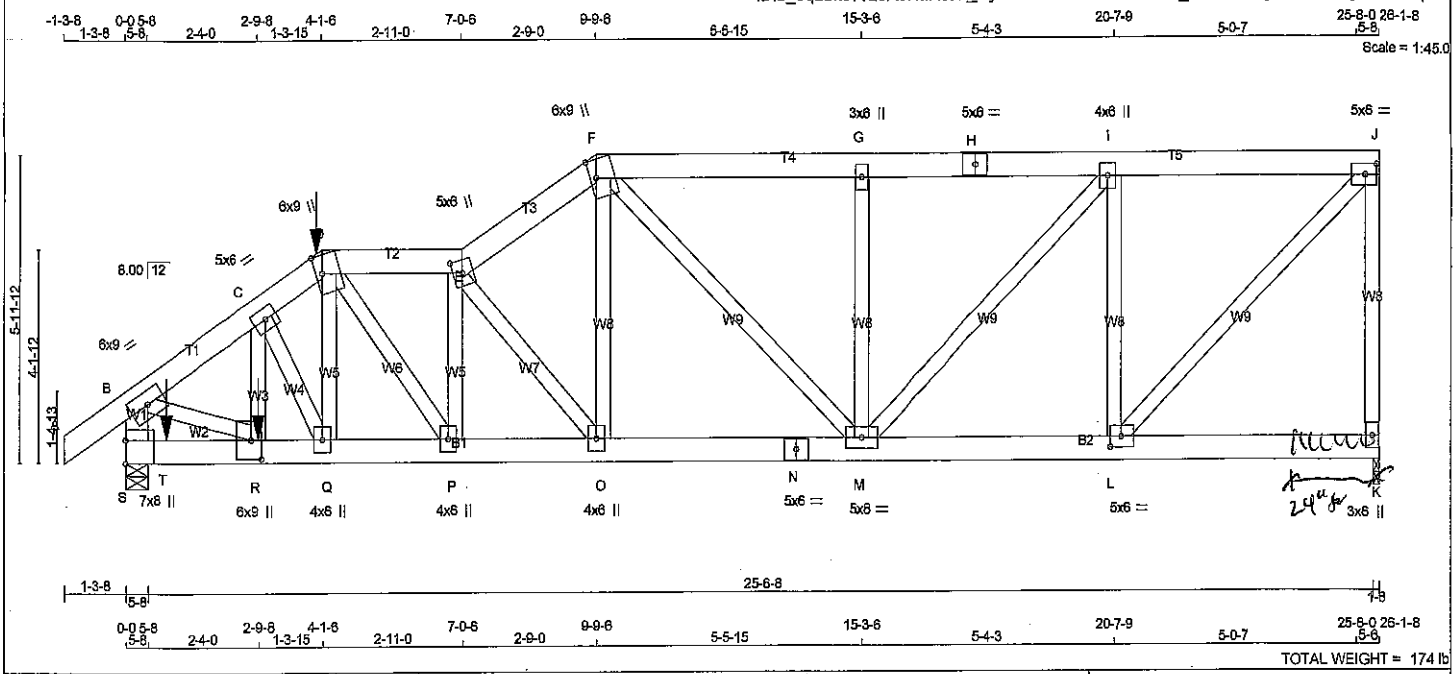

TOTAL WEIGHT = 174 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - E	2x6	DRY	No.2	SPF
E - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
S - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	6.0	9.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	6.0	9.0	4.25	1.50
E	TTWW+m	MT20	5.0	6.0	3.00	2.25
F	TTWW+m	MT20	6.0	9.0	4.25	1.50
G	TMW+w	MT20	3.0	6.0		
H	TS-t	MT20	5.0	6.0		
I	TMWW+t	MT20	4.0	6.0		
J	TMVW-t	MT20	5.0	6.0	2.50	2.75
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.75
M	BMWWW-t	MT20	5.0	8.0		
N	BS-t	MT20	5.0	6.0		
O, P, Q						
O	BMWW+t	MT20	4.0	6.0		
R	BMWW+t	MT20	6.0	8.0	4.50	2.50
S	BMV1+t	MT20	7.0	8.0	5.50	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 254.8 lbs FACTORED DOWN AT 4-1-6 ON TOP CHORD, AND 75.1 lbs FACTORED DOWN AT 10-4, AND 2102.9 lbs FACTORED DOWN AT 2-9-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	1733	0	1733	0	0	1-8	1-8
S	3720	0	3720	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	1414	798 / 0	313 / 0	0 / 0	0 / 0	303 / 0	0 / 0
S	2999	1754 / 0	623 / 0	0 / 0	0 / 0	622 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, S

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.11 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	FORCE (LBS)	MAX CSI (LC)
FR-TO		(PLF)						
A- B	0 / 33	-84.3	-84.3	0.07 (1)	10.00	R- C	0 / 808	0.14 (1)
B- C	-4014 / 0	-84.3	-84.3	0.21 (1)	4.11	C- Q	-724 / 0	0.10 (1)
C- D	-3539 / 0	-84.3	-84.3	0.17 (1)	4.38	Q- D	0 / 978	0.17 (1)
D- E	-3399 / 0	-84.3	-84.3	0.12 (1)	4.50	D- P	0 / 640	0.11 (1)
E- F	-2876 / 0	-84.3	-84.3	0.10 (1)	4.85	P- E	-535 / 0	0.09 (1)
F- G	-2254 / 0	-84.3	-84.3	0.20 (1)	5.22	E- O	-1657 / 0	0.41 (1)
G- H	-2254 / 0	-84.3	-84.3	0.20 (1)	5.22	O- F	0 / 1456	0.26 (1)
H- I	-2254 / 0	-84.3	-84.3	0.20 (1)	5.22	B- R	0 / 3482	0.62 (1)
I- J	-1469 / 0	-84.3	-84.3	0.18 (1)	6.17	L- J	0 / 2082	0.37 (1)
K- J	-1667 / 0	0.0	0.0	0.98 (1)	6.38	F- M	-251 / 0	0.20 (1)
S- B	-3566 / 0	0.0	0.0	0.25 (1)	5.60	L- I	-1309 / 0	0.47 (1)
						M- G	-484 / 0	0.17 (1)
S- T	0 / 0	-28.0	-28.0	0.18 (1)	10.00	M- I	0 / 1128	0.20 (1)
T- R	0 / 0	-28.0	-28.0	0.18 (1)	10.00			
R- Q	0 / 3315	-28.0	-28.0	0.61 (1)	10.00			
Q- P	0 / 3013	-28.0	-28.0	0.45 (1)	10.00			
P- O	0 / 3420	-28.0	-28.0	0.51 (1)	10.00			
O- N	0 / 2432	-28.0	-28.0	0.36 (1)	10.00			
N- M	0 / 2432	-28.0	-28.0	0.36 (1)	10.00			
M- L	0 / 1469	-28.0	-28.0	0.24 (1)	10.00			
L- K	0 / 0	-28.0	-28.0	0.10 (3)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
D	4-1-6	-24	-27	—	FRONT	VERT	DEAD
D	4-1-6	-231	-231	—	FRONT	VERT	SNOW
R	2-9-8	-2103	-2103	—	FRONT	VERT	TOTAL
T	10-4	-43	-75	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 8.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.87")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.87")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.98/1.00 (J-K:1) , BC=0.61/1.00 (Q-R:1) , WB=0.62/1.00 (B-R:1) , SSI=0.18/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1687	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

LICENSED PROFESSIONAL ENGINEER
2018
S. KATSOULAKOS
PROVINCE OF ONTARIO

SCAB JOINT # 1 & #__ WITH 2 X 6 #2 SPF 24" LONG 1 SIDE(S) USING 2 ROW(S) 3½" ~~ARDOX~~ SPIRAL/COMMON WIRE NAILS AT 3" O/C STAGGERED (15 NAILS/SCAB). REQUIRED FOR 1½" BRG PROVISION.

DWG NO. TAM 8303-18
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285415	T170S	1	1	TRUSS DESC.	

JSI GRIP= 0.88 (R) (INPUT = 0.90)
JSI METAL= 0.72 (R) (INPUT = 1.00)

DWG NO. TAM 8302-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285415	T180S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Dec 12 2017 MiTek Industries, Inc. Tue Feb 6 16:55:13 2018 Page 1

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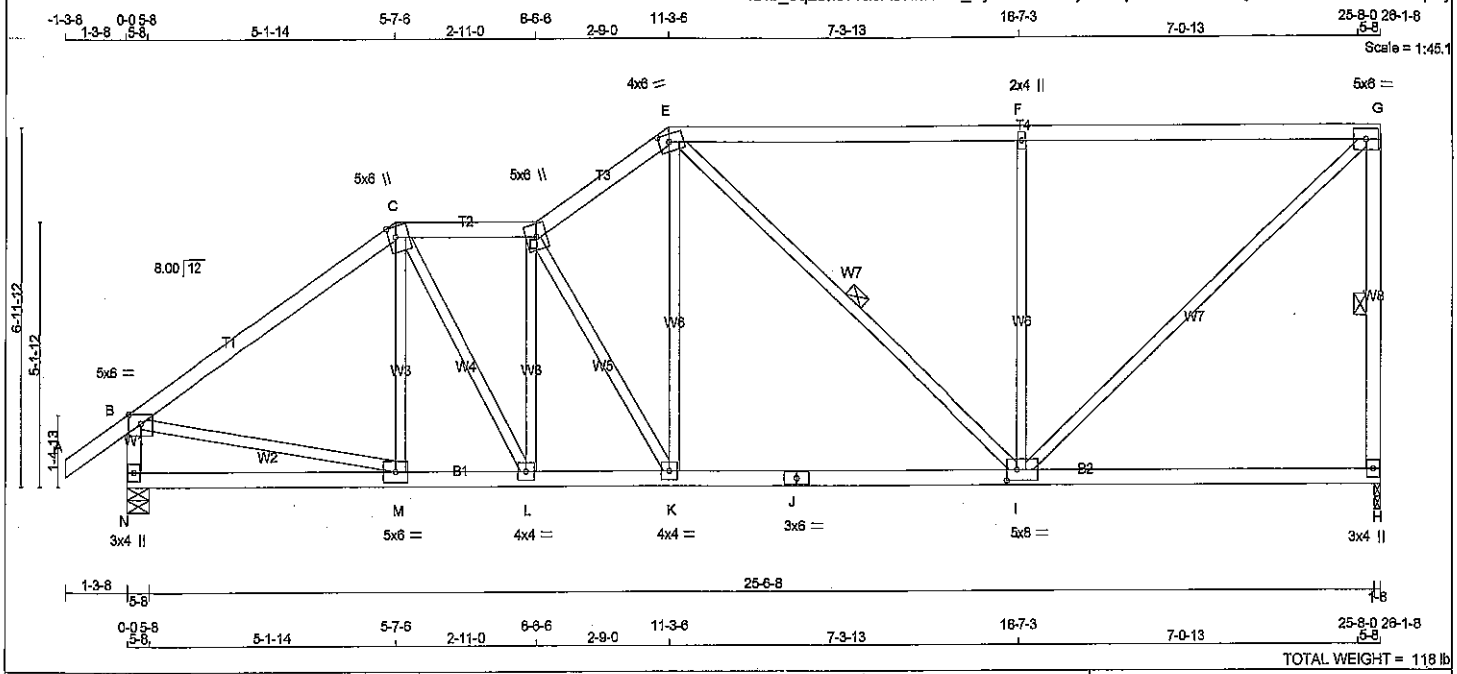

TOTAL WEIGHT = 118 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	Edge	1.75
D	TTWW+m	MT20	5.0	6.0		
E	TTWW-m	MT20	4.0	6.0	1.75	2.50
F	TMW+w	MT20	2.0	4.0		
G	TMVW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWWW-t	MT20	5.0	8.0	2.50	2.50
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMWW-t	MT20	5.0	6.0		
N	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1466	0	1466	0	0	1-8	1-8
N	1582	0	1582	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1204	668 / 0	274 / 0	0 / 0	0 / 0	261 / 0	0 / 0
N	1282	739 / 0	274 / 0	0 / 0	0 / 0	269 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-H, E-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	M- C	-82 / 166	0.04 (3)
B- C	-1632 / 0	-84.3	-84.3	0.58 (1)	4.50	C- L	0 / 729	0.16 (1)
C- D	-1721 / 0	-84.3	-84.3	0.16 (1)	4.93	L- D	-630 / 0	0.25 (1)
D- E	-1895 / 0	-84.3	-84.3	0.14 (1)	4.97	D- K	-638 / 0	0.34 (1)
E- F	-1277 / 0	-84.3	-84.3	0.93 (1)	4.00	K- E	0 / 718	0.16 (1)
F- G	-1278 / 0	-84.3	-84.3	0.93 (1)	4.00	E- I	-194 / 0	0.14 (1)
H- G	-1383 / 0	0.0	0.0	0.31 (1)	5.53	I- F	-775 / 0	0.65 (1)
N- B	-1516 / 0	0.0	0.0	0.16 (1)	6.67	I- G	0 / 1716	0.39 (1)
						B- M	0 / 1361	0.31 (1)
N- M	0 / 0	-28.0	-28.0	0.20 (3)	10.00			
M- L	0 / 1355	-28.0	-28.0	0.36 (2)	10.00			
L- K	0 / 1735	-28.0	-28.0	0.39 (2)	10.00			
K- J	0 / 1422	-28.0	-28.0	0.57 (2)	10.00			
J- I	0 / 1422	-28.0	-28.0	0.57 (2)	10.00			
I- H	0 / 0	-28.0	-28.0	0.40 (3)	10.00			

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.87")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.87")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.93/1.00 (F-G:1) , BC=0.57/1.00 (I-K:2) , WB=0.65/1.00 (F-I:1) , SSI=0.30/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

LS GRIP= 0.89 (B) (INPUT = 0.90)
LS METAL= 0.50 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER 2016 S KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM B3048
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285415	T190S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Dec 12 2017 MiTek Industries, Inc. Tue Feb 6 16:55:16 2018 Page 1
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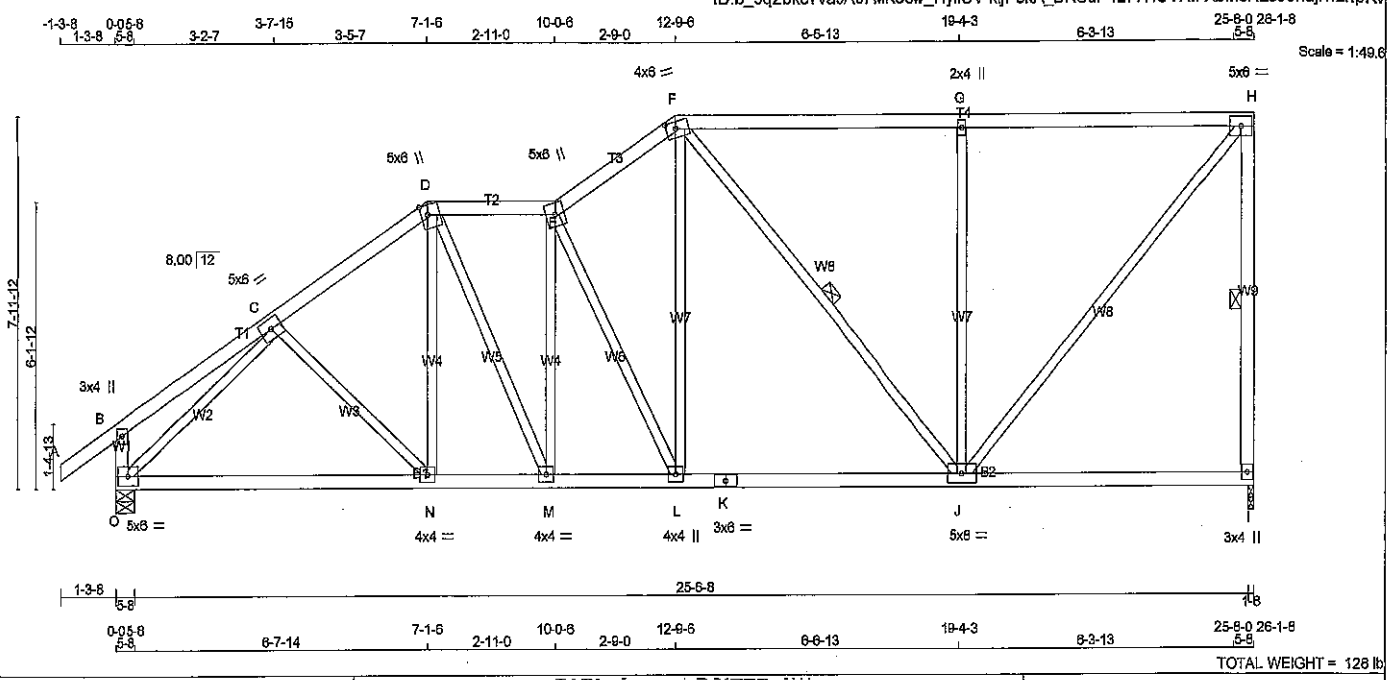


TOTAL WEIGHT = 126 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	6.0	Edge	1.75
E	TTWW+m	MT20	5.0	6.0		
F	TTWW-m	MT20	4.0	6.0	1.75	2.50
G	TMW+w	MT20	2.0	4.0		
H	TMVW-t	MT20	5.0	6.0		
I	BMV1+p	MT20	3.0	4.0		
J	BMWWW-t	MT20	5.0	8.0		
K	BS-t	MT20	3.0	6.0		
L	BMWW+t	MT20	4.0	4.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMVW1-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	1466	0	1466	0	0	1-8	1-8
O	1582	0	1582	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1204	668 / 0	274 / 0	0 / 0	0 / 0	261 / 0	0 / 0
O	1282	739 / 0	274 / 0	0 / 0	0 / 0	269 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF H-I, F-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00
B- C	0 / 17	-84.3	-84.3	0.15 (1)	10.00
C- D	-1608 / 0	-84.3	-84.3	0.19 (1)	5.02
D- E	-1538 / 0	-84.3	-84.3	0.15 (1)	5.16
E- F	-1498 / 0	-84.3	-84.3	0.14 (1)	5.22
F- G	-1027 / 0	-84.3	-84.3	0.70 (1)	5.00
G- H	-1028 / 0	-84.3	-84.3	0.70 (1)	5.00
I- H	-1392 / 0	0.0	0.0	0.40 (1)	5.52
O- B	-234 / 0	0.0	0.0	0.02 (1)	7.81
O- N	0 / 1303	-28.0	-28.0	0.47 (2)	10.00
N- M	0 / 1325	-28.0	-28.0	0.49 (2)	10.00
M- L	0 / 1546	-28.0	-28.0	0.34 (2)	10.00
L- K	0 / 1257	-28.0	-28.0	0.47 (2)	10.00
K- J	0 / 1257	-28.0	-28.0	0.47 (2)	10.00
J- I	0 / 0	-28.0	-28.0	0.32 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- N	0 / 130	0.03 (3)
N- D	0 / 305	0.07 (2)
D- M	0 / 490	0.11 (1)
M- E	-457 / 0	0.27 (1)
E- L	-882 / 0	0.52 (1)
L- F	0 / 768	0.17 (1)
F- J	-351 / 0	0.26 (1)
J- G	-697 / 0	0.84 (1)
J- H	0 / 1562	0.35 (1)
O- C	-1817 / 0	0.71 (1)

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.87")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.87")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.70/1.00 (G-H:1) , BC=0.49/1.00 (M-N:2) , WB=0.84/1.00 (G-J:1) , SSI=0.27/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

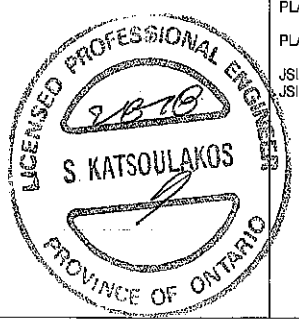
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (J) (INPUT = 0.90)
JSI METAL= 0.45 (C) (INPUT = 1.00)



DWG NO. TAM 0305 -18
STRUCTURAL COMPONENT ONLY

TECHNICAL BULLETIN

LUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

MATERIAL: 18 gauge

FINISH: G90 galvanized

DESIGN:

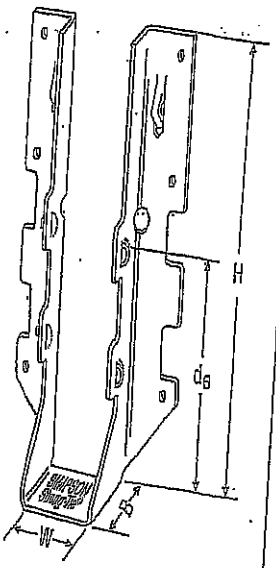
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

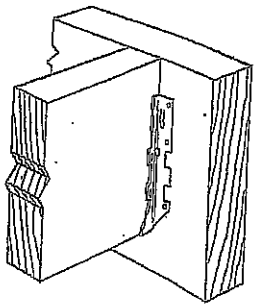
INSTALLATION:

- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

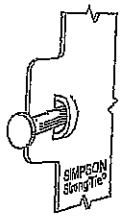
- These hangers cannot be modified.




Typical LUS Installation

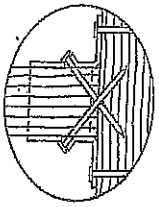
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
								D.Fir-L		S-P-F	
		W	H	B	d_B[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.60)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LUS24	18	1⅜	3⅛	1¾	1⅞	4-10d	2-16d	710	1680	645	1165
LUS24-2	18	3⅛	3⅛	2	1⅞	4-16d	2-16d	885	2020	590	1435
LUS26	18	1⅜	4¾	1¾	3⅝	4-10d	4-10d	1420	2170	1290	1630
LUS26-2	18	3⅛	4⅞	2	4	4-16d	4-16d	1720	2695	1545	1920
LUS26-3	18	4⅝	4⅞	2	3¼	4-16d	4-16d	1720	2695	1545	2940
LUS28	18	1⅜	6⅝	1¾	3¾	6-10d	4-10d	1420	2520	1290	1790
LUS28-2	18	3⅛	7	2	4	6-16d	4-16d	1720	3825	1545	2575
LUS28-3	18	4⅝	6¼	2	3¼	6-16d	4-16d	1720	3825	1545	2875
LUS210	18	1⅜	7⅞	1¾	3⅝	8-10d	4-10d	1420	2785	1290	2210
LUS210-2	18	3⅛	9	2	6	8-16d	6-16d	2580	4500	2320	3195
LUS210-3	18	4⅝	8⅞	2	5¼	8-16d	6-16d	2580	3845	2320	2875

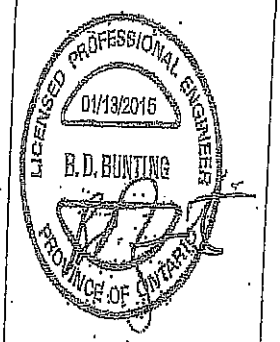
1. d_B is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580


Double Shear Nailing Top View.



LIMIT STATES DESIGN

TECHNICAL BULLETIN

HUS/LJS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: See table

FINISH: G90 galvanized

DESIGN:

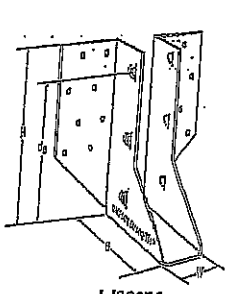
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15% No further increase is permitted
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

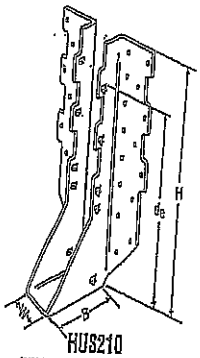
INSTALLATION:

- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

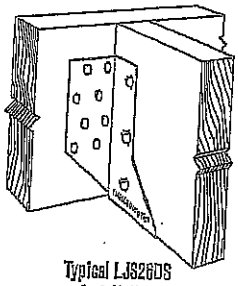
OPTIONS:

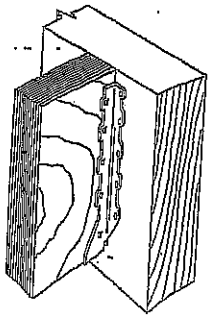
- See current catalogue for options

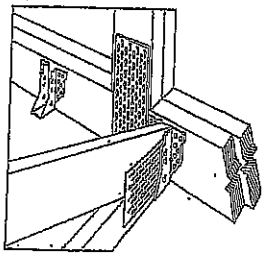

LJS26DS



HUS210
(HUS26, HUS28, similar)

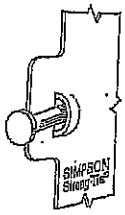

Typical LJS26DS Installation


Typical HUS Installation

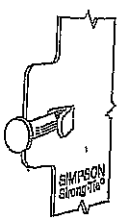

Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

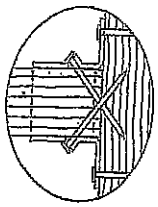
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
								D.Fir-L.		S-P-F	
		W	H	B	d_s[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LJS26DS	18	1⅞	6	3½	4⅝	16-16d	6-16d	2065	4265	1460	4115
HUS26	16	1⅞	5⅝	3	3¹⁵⁄₁₆	14-16d	6-16d	2705	4940	2065	3875
HUS28	16	1⅞	7⅝	3	5⅝	22-16d	8-16d	3605	6365	2675	4845
HUS210	16	1⅞	9⅝	3	7⅝	30-16d	10-16d	4505	5795	4010	4740
HUS1.81/10	16	1⅞	9	3	8	30-16d	10-16d	4505	6450	4010	5200

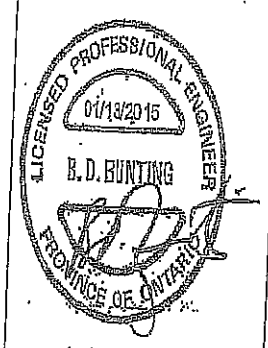
1. d_s is the distance from the seat of the hanger to the highest joist nail.


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580


Double Shear Nailing Side View. Do not bend tab back.


Double Shear Nailing Top View.



HGUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: 12 gauge

FINISH: G90 galvanized

DESIGN:

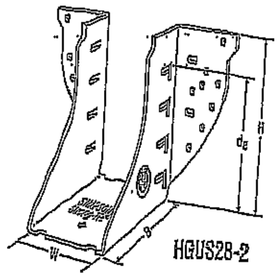
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

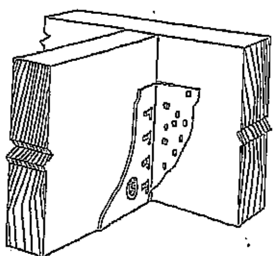
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

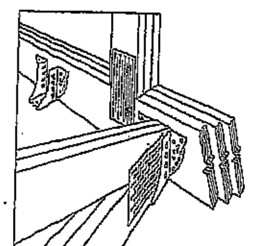
OPTIONS:

- See current catalogue for options.



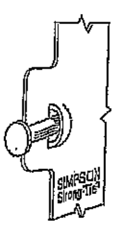
HGUS28-2


Typical HGUS Installation


Typical HGUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)

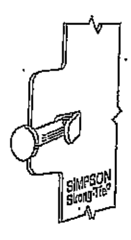
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
HGUS26	12	1⅝	5⅜	5	4⁵⁄₃₂	20-16d	8-16d	2685	6625	2685	5700
HGUS26-2	12	3⁵⁄₁₆	5⁷⁄₁₆	4	4⅛	20-16d	8-16d	4385	8950	3100	6355
HGUS26-3	12	4¹⁵⁄₁₆	5½	4	4⅛	20-16d	8-16d	4385	8950	3100	6355
HGUS26-4	12	6⁹⁄₁₆	5⁷⁄₁₆	4	4⅛	20-16d	8-16d	4385	8950	3100	6355
HGUS28	12	1⅝	7⅛	5	6⅛	36-16d	12-16d	3310	7675	3100	6900
HGUS28-2	12	3⁵⁄₁₆	7⁹⁄₁₆	4	6⅛	36-16d	12-16d	6070	12980	4310	9215
HGUS28-3	12	4¹⁵⁄₁₆	7¼	4	6⅜	36-16d	12-16d	6070	12980	4310	9215
HGUS28-4	12	6⁹⁄₁₆	7⁹⁄₁₆	4	6⅛	36-16d	12-16d	6070	12980	4310	9215
HGU210-2	12	3⁵⁄₁₆	9⁹⁄₁₆	4	8⅛	46-16d	16-16d	6840	14645	4855	10400
HGUS210-3	12	4¹⁵⁄₁₆	9¼	4	8⅜	46-16d	16-16d	6840	14645	4855	10400
HGUS210-4	12	6⁹⁄₁₆	9⁹⁄₁₆	4	8⅛	46-16d	16-16d	6840	14645	4855	10400
HGUS212-4	12	6⁹⁄₁₆	10⅝	4	10⅛	56-16d	20-16d	7640	14995	5425	10645
HGUS214-4	12	6⁹⁄₁₆	12⅝	4	11⅛	66-16d	22-16d	10130	16400	7195	11645

1. d_e is the distance from the seat of the hanger to the highest joist nail.

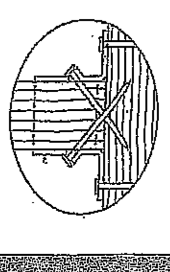


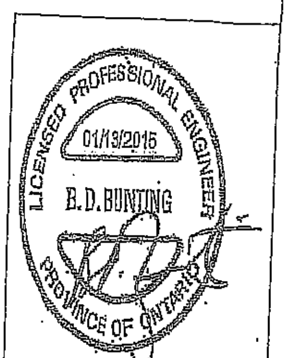
Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.

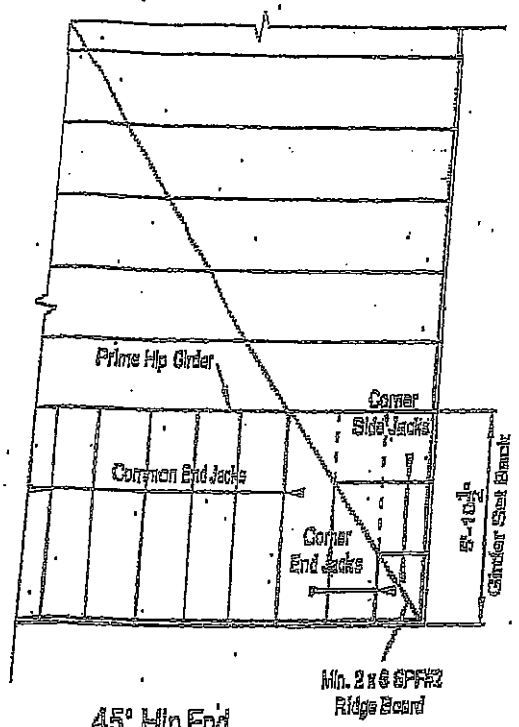

Double Shear Nailing Top View.



 T-SPECHGUS15 2/15 exp. 12/16

MICRO CITY
ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242



45° Hip End

LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2

BOTTOM CHORD : 2 x 4 SPF#2

WEBS : 2 x 3 SPF#2

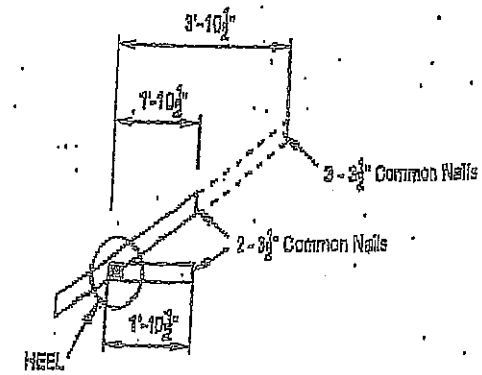
UNLESS OTHERWISE SHOWN

DESIGN LOAD:

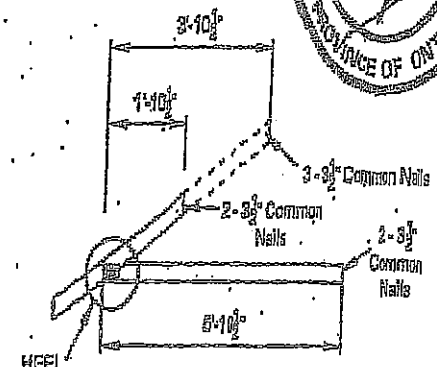
TOP CHORD LIVE LOAD	:	39.4 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	10.5 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F
TOTAL LOAD		59.9 P.S.F.

DWG NO THIN 2475.14
STRUCTURAL
COMPONENT ONLY

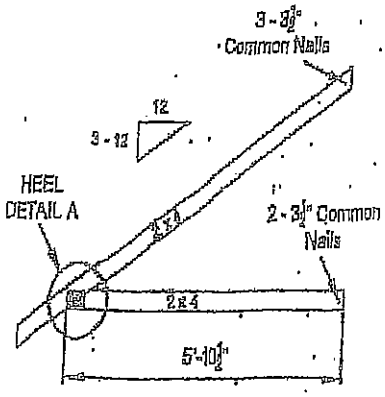
LICENCED PROFESSIONAL ENGINEER
1-20-14
S. KATSOULAKOS
PROVINCE OF ONTARIO



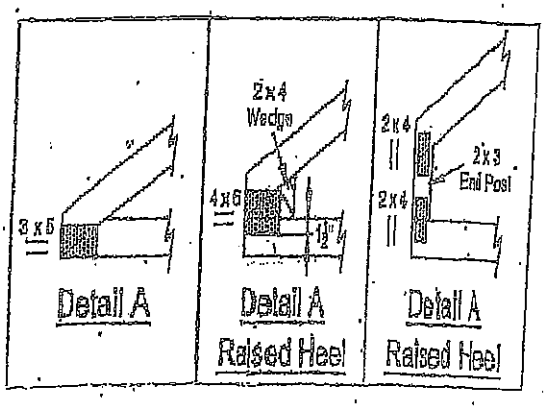
Corner Side Jacks



Corner End Jacks



Common End Jacks



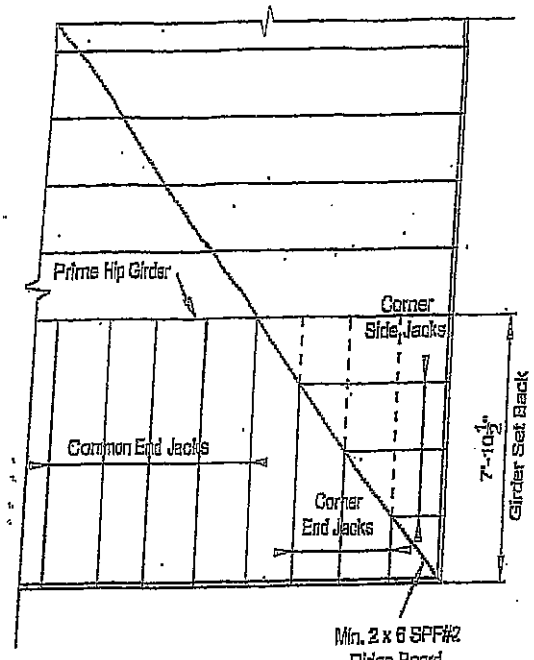
Detail A | Detail A Raised Heel | Detail A Raised Heel

NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)

MICRO CITY
ENGINEERING SERVICES INC.
TEL: (519) 287.- 2242
R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

LUMBER SPECIFICATION

TOP CHORD : 2x4 SPF#2
BOTTOM CHORD : 2x4 SPF#2
WEBS : 2x3 SPF#2
UNLESS OTHERWISE SHOWN



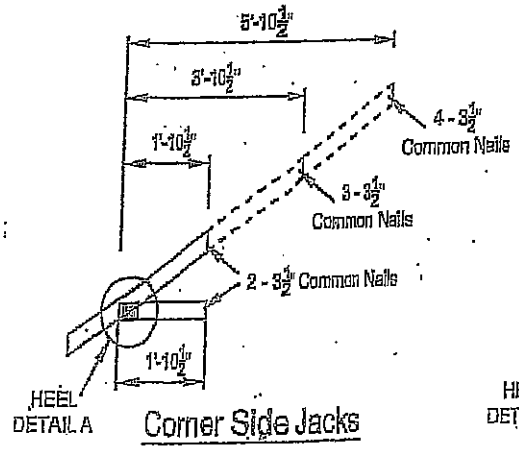
45° Hip End

DESIGN LOAD:

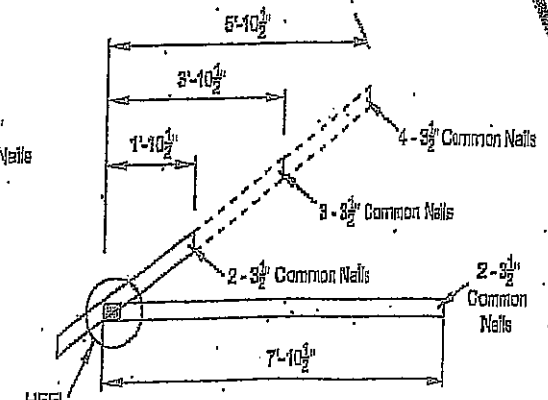
TOP CHORD LIVE LOAD	: 34.8 P.S.F.
TOP CHORD DEAD LOAD	: 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	: 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	: 7.0 P.S.F.
TOTAL LOAD	: 44.8 P.S.F.

DWG NO TRM 3503 14
STRUCTURAL
COMPONENT ONLY

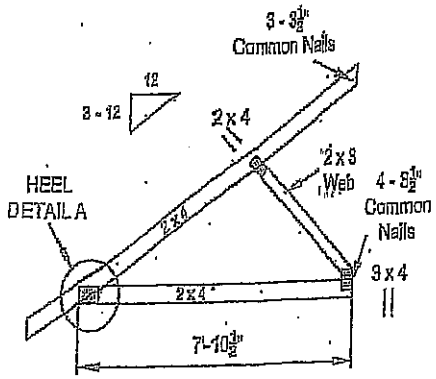
LICENSED PROFESSIONAL ENGINEER
1-2014
S. KATSOULAKOS
PROVINCE OF ONTARIO



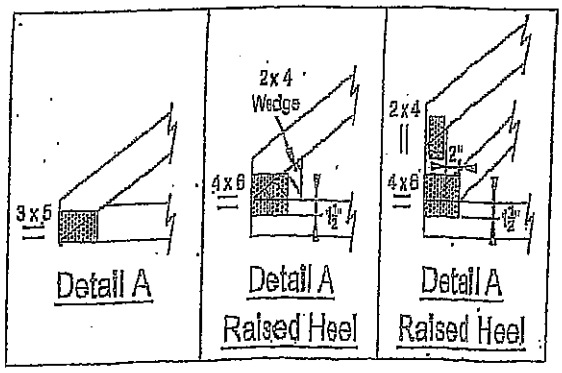
Corner Side Jacks



Corner End Jacks



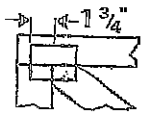
Common End Jacks



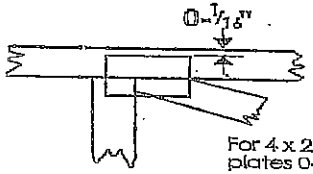
NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/8" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

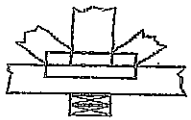
4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

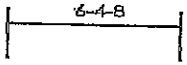
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses

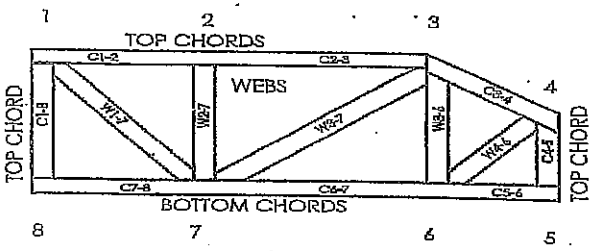
DSB-89: Design Standard for Bracing.

BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

MiTek®

POWER TO PERFORM.™

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

Micro City Engineering Services Inc.
(BCIN: 26064; FIRM BCIN: 29991)
RR #1, Po Box 61
Glencoe, Ontario
N0L 1M0
(519) 287 - 2242; Fax: (519) 287 - 5750 (Call)

LICENSED PROFESSIONAL ENGINEER
1-2014
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO [illegible] 1-24-14
STRUCTURAL
COMPONENT ONLY

Responsibilities:

Micro City Engineering Services is responsible for the design of trusses as individual components.

It is the responsibilities of others to ascertain that the design loads utilized on this (these) drawing(s) meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdiction over such decisions.

All dimensions are to be verified by the owner, contractor, architect, or other authority having input over such decisions prior to truss component manufacture. At no time shall Micro City Engineering Services Inc. or its employees be responsible for dimension errors.

Micro City Engineering Services Inc. bears no responsibility for the erection of any truss components. Persons erecting truss components are cautioned to seek professional advice regarding temporary and permanent bracing systems and to be totally familiar with all aspects of truss erection prior to proceeding on any truss component erection job. Any bracing shown on Micro City Engineering Services Inc. or Tamarack Roof Trusses Inc. sealed or unsealed truss component drawings is specified for the single truss component in question and is identified as an integral part of the design for that particular truss component but is not meant to represent the only required bracing for that particular truss component when installed as a component in a series of truss components in a roof truss system.

~~It is the truss manufacturer's responsibility to ensure that trusses are manufactured in accordance with Micro City Engineering~~ Services Inc. specifications outlined below:

SPECIFICATIONS:

Truss components sealed by Micro City Engineering Services Inc. must conform to the relevant sections of the current Building Code of Ontario and Canada (Part 4 or Part 9) or the current Farm Building Code of Canada in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the Truss Plate Institute of Canada (TPIC). All unit lumber and nailing stresses identified on truss component design drawings and/or used in the design of individual truss components shall conform to the current CSA Wood Design standard identified in the current Building Code and TPIC Design Standards.

The lumber used to manufacture any truss component is to conform to the specified size and grade identified on the truss drawing.

The lumber used in the manufacture of any truss component is not to exceed 19% during its service use unless specifically noted on the truss drawing.

The lumber used in the manufacture of any truss component is not to be treated with any chemicals during its service life unless specifically noted on the truss drawing.

Connector plates shall be applied to both faces of the truss component at each joint and shall be positioned exactly as specified.

The top chord of any truss component is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the sealed truss component drawing but not exceeding 24" o/c (Part 9 design) and not exceeding 48" o/c (Part 4 or Agricultural design).

When a truss component is to be installed with no rigid ceiling attached directly to the bottom chord, then the bottom chord is to be laterally braced at intervals not exceeding 3m (or 10'-0").

All sealed or unsealed truss component drawings provided by Micro City Engineering Services Inc. Or Tamarack Roof Trusses Inc. should be read in conjunction with the following:

Warning-Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473C rev 10-'08 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer - not the truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component Safety Information available from the Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA, 22314.