ASPHALT SHINGLES
12"FINISH O.H.
R.T.M.C
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:

HGUS26-2 -(XX)
LJS26DS -(V)

8/12 PITCHES (TYP.)
UNLESS NOTED

DENOTES:

✱ -2'-0-0 HIGHER PLATE

H -1'-6"-0 RAISED CEILING

ALL B- 2-2X10 BEAMS (FLUSH)

HIGHGROVE-12
EL:1 - UNIT1

285930

HIGHGROVE-1
EL:1 - UNIT2

285931

HIGHGROVE-2
EL:1(REV) - UNIT3

285932

HIGHGROVE-2
EL:1 - UNIT4

285298

HIGHGROVE-5E
EL:1(REV) - UNIT5

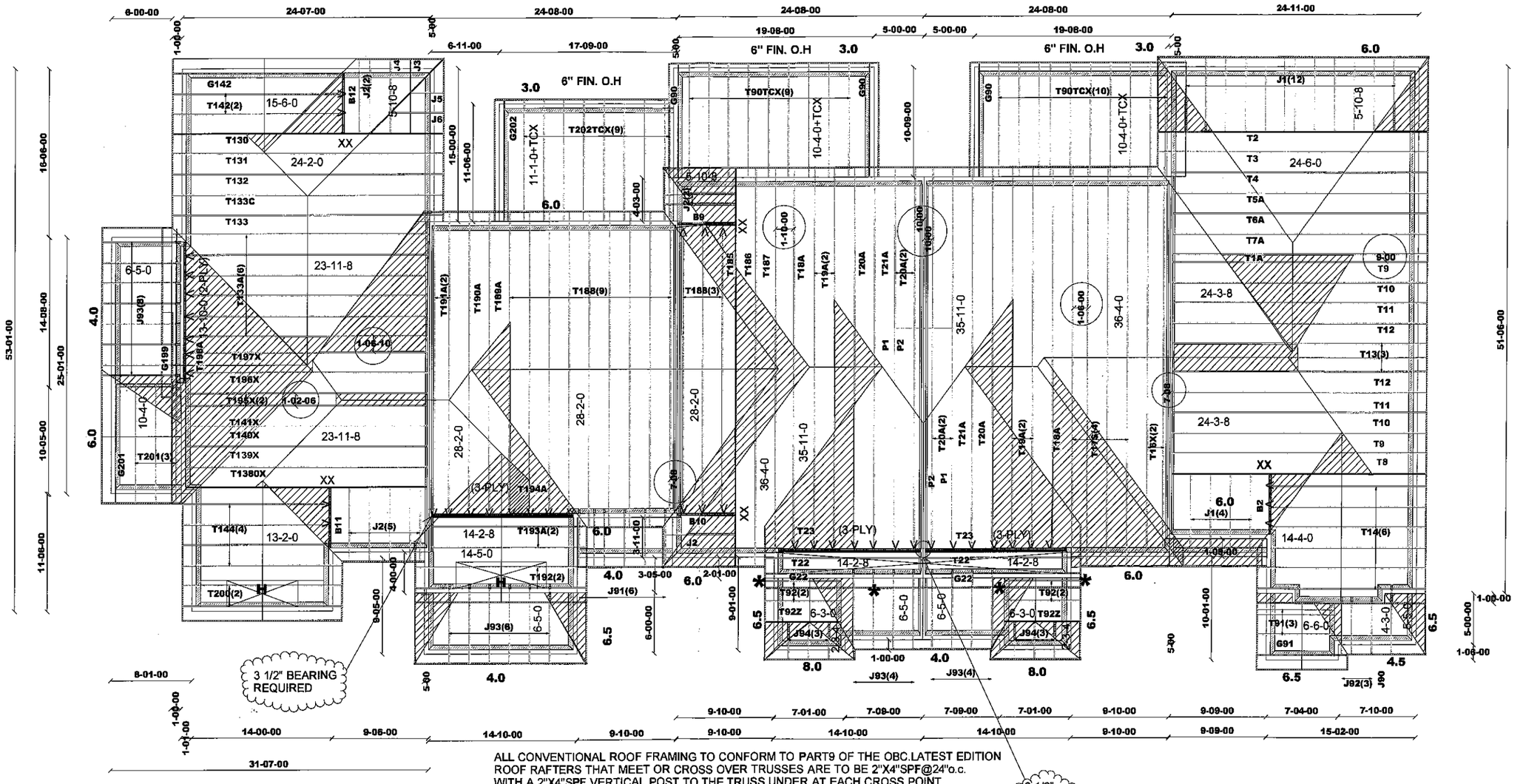
285934

1 '-4"-0 WALL DIFF. C OF WOOD PARTY WALL ABOVE CEILING

C OF WOOD PARTY WALL ABOVE CEILING

C OF WOOD PARTY WALL ABOVE CEILING

C OF WOOD PARTY WALL ABOVE CEILING



ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION
ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"o.c.
WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.
POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE
BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN LOADS:

SNOW LOAD 25.6 PSF
TC DEAD 3 PSF
BC LIVE 10.5 PSF

T-170534

DENOTES:
CONVENTIONAL
FRAMING

m11,008

TAMARACK LUMBER INC. ALPA LUMBER GROUP

Job Track: 43954	Builder / Location: GREENPARK HOMES / WATERDOWN	Model / Elevation: BLOCK216/ UNIT1TO5 (REVISED)	Mitek ver 7.5.0
Layout ID: 285929	Project: RUSSEL GARDENS PHASE 2	THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTLILZED FOR ANY OTHER PURPOSE.	
Plan Log: 93446	Date: 4/17/2018 Designer: sonny		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

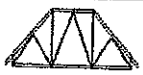
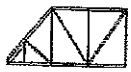
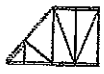
Delivery Shiplist

DATE	09/08/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 285930 LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH 2 SUB-BUILDER:
MODEL: HIGHGROVE 12 ELEVATION: 1- UNIT1

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T130 HIP GIRDER	8.00 0.00	24-02-00	05-03-13	2 X 6	2 X 6	01-03-08 01-03-08	01-04-13 01-04-13	144.06 87.67		
	1	T131 HIP	8.00 0.00	24-02-00	06-07-13	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	103.24 67.00		
	1	T132 HIP	8.00 0.00	24-02-00	07-11-13	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	109.56 69.50		
	1	T133 COMMON	8.00 0.00	24-02-00	09-05-08	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	103.85 64.17		
	1	T133C COMMON	8.00 0.00	24-02-00	09-03-13	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	103.67 64.17		
	6	T133A COMMON	8.00 0.00	23-11-08	09-05-08	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	609.42 381.00		
	1	T1380X HALF HIP	8.00 0.00	23-11-08	05-03-13	2 X 6	2 X 6	00-00-00 00-00-00	00-08-02 05-03-13	145.78 87.00		
	1	T139X HALF HIP	8.00 0.00	23-11-08	06-07-13	2 X 4	2 X 4	00-00-00 00-00-00	00-08-02 06-07-13	103.32 65.17		
	1	T140X HALF HIP	8.00 0.00	23-11-08	07-11-13	2 X 4	2 X 4	00-00-00 00-00-00	00-08-02 07-11-13	111.52 70.00		
	1	T141X HALF HIP	8.00 0.00	23-11-08	09-03-13	2 X 4	2 X 4	00-00-00 00-00-00	00-08-02 09-03-13	122.11 74.83		
	2	T195X HALF HIP	8.00 0.00	23-11-08	10-07-13	2 X 4	2 X 4	00-00-00 00-00-00	00-08-02 10-07-13	251.36 153.00		
	1	T196X HALF HIP	8.00 0.00	23-11-08	09-07-13	2 X 4	2 X 4	00-00-00 00-00-00	00-08-02 09-07-13	130.78 81.33		
	1	T197X HALF HIP	8.00 0.00	23-11-08	08-07-13	2 X 4	2 X 4	00-00-00 00-00-00	00-08-02 08-07-13	114.71 71.00		
	1 2 Ply	T198A COMMON	8.00 0.00	13-10-00	09-05-13	2 X 4	2 X 6	00-00-00 00-00-00	01-04-13 08-04-02	198.74 119.34		
	1	G199 COMMON	8.00 0.00	13-10-00	06-00-02	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	58.80 38.33		
	2	T142 COMMON	8.00 0.00	15-06-00	06-06-13	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	132.28 84.00		
	1	G142 GABLE	8.00 0.00	15-06-00	06-06-13	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	66.17 42.00		
	2	T200 ROOF	8.00 7.00	13-02-00	05-09-08	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	115.36 75.34		

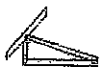
DATE	09/08/17
SALES REP	Mario

Delivery Shiplist

JOB TRACK: 43954 LAYOUT ID: 285930 LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH 2 SUB-BUILDER:
MODEL: HIGHGROVE 12 ELEVATION: 1- UNIT1

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	4	T144 COMMON	8.00 0.00	13-02-00	05-09-08	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	229.52 148.00		
	3	T201 COMMON	6.00 0.00	10-04-00	03-01-12	2 X 4	2 X 4	01-03-08 01-03-08	00-06-12 00-06-12	95.94 62.49		
	1	G201 GABLE	6.00 0.00	10-04-00	03-01-12	2 X 4	2 X 4	01-03-08 01-03-08	00-06-12 00-06-12	34.56 22.83		
	7	J2 JACK-OPEN	8.00 0.00	05-10-08	05-03-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-03-13	124.81 74.69		
	1	J3 JACK-OPEN	8.00 0.00	05-10-08	02-07-02	2 X 4	2 X 4	01-03-08 -04-01-01	01-04-13 00-03-08	16.84 10.33		
	1	J4 JACK-OPEN	8.00 0.00	05-10-08	03-11-02	2 X 4	2 X 4	01-03-08 -02-01-01	01-04-13 00-03-08	19.58 12.33		
	1	J5 JACK-OPEN	8.00 0.00	01-10-08	02-07-02	2 X 4	2 X 4	01-03-08 -00-01-01	01-04-13 00-03-08	9.17 5.67		
	1	J6 JACK-OPEN	8.00 0.00	01-10-08	03-11-02	2 X 4	2 X 4	01-03-08 01-10-15	01-04-13 00-05-03	11.91 7.67		
	8	J93 MONOPITCH	4.00 0.00	06-05-00	02-05-10	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-05-10	145.28 93.36		

TOTAL # TRUSS= 54.00 TOTAL BFT OF ALL TRUSSES= 2132.22 BFT. TOTAL WEIGHT OF ALL TRUSSES= 3412.34 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
11	Hangers	LJS26DS	

TOTAL # ITEMS= 13.00

TAMARACK

Delivery Shiplist

DATE	09/07/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 285931 LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH 3 SUB-BUILDER:
MODEL: HIGHGROVE 1 ELEVATION: 1-UNIT2

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	9	T188 COMMON	6.00 / 0.00	28-02-00	08-02-08	2 X 4	2 X 4	01-03-08 / 01-03-08	01-02-00 / 01-02-00	991.44 / 618.03		
	1	T189A ROOF	6.00 / 0.00	28-02-00	06-11-13	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 02-03-08	122.06 / 77.00		
	1	T190A ROOF	6.00 / 0.00	28-02-00	08-03-13	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 02-03-08	119.13 / 74.67		
	2	T191A ROOF	6.00 / 0.00	28-02-00	09-08-06	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 02-03-08	235.82 / 147.34		
	2	T192 ROOF	8.00 / 0.00	14-05-00	06-02-08	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	137.12 / 89.00		
	2	T193A COMMON	8.00 / 0.00	14-02-08	06-02-08	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 01-06-08	119.62 / 76.66		
	1 / 3 Ply	T194A COMMON	8.00 / 0.00	14-02-08	06-02-08	2 X 6	2 X 6	00-00-00 / 00-00-00	01-04-13 / 01-06-08	256.56 / 159.99		
	9	T202TCX MONOPITCH	3.00 / 0.00	11-01-00	03-07-13	2 X 6	2 X 4	00-09-08 / 00-00-00	00-08-04 / 03-07-13	431.37 / 279.00		
	1	G202 GABLE	3.00 / 0.00	11-01-00	03-05-08	2 X 6	2 X 4	00-09-08 / 00-00-00	00-08-04 / 03-05-08	43.96 / 29.33		
	6	J91 MONOPITCH	4.00 / 0.00	04-04-00	01-09-04	2 X 4	2 X 4	01-03-08 / 00-00-00	00-03-15 / 01-09-04	70.86 / 48.00		
	6	J93 MONOPITCH	4.00 / 0.00	06-05-00	02-05-10	2 X 4	2 X 4	01-03-08 / 00-00-00	00-03-15 / 02-05-10	108.96 / 70.02		

TOTAL # TRUSS= 42.00 TOTAL BFT OF ALL TRUSSES= 1669.04 BFT. TOTAL WEIGHT OF ALL TRUSSES= 2636.90 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
8	Hangers	LJS26DS	

TOTAL # ITEMS= 8.00

TAMARACK LUMBER INC.

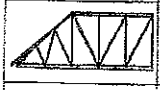
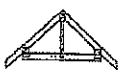
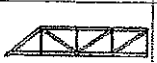
Delivery Shiplist

DATE	09/07/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 285932 LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH 2 SUB-BUILDER:
MODEL: HIGHGROVE 2 ELEVATION: 1- (REV)- UNIT3

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T185 HIP GIRDER	6.00 0.00	36-04-00	05-03-13	2 X 4	2 X 6	01-03-08 01-03-08	01-02-00 01-02-00	215.70 129.67		
	1	T186 HIP	6.00 0.00	36-04-00	06-07-13	2 X 4	2 X 6	01-03-08 01-03-08	01-02-00 01-02-00	168.08 103.67		
	1	T187 HIP	6.00 0.00	36-04-00	07-11-13	2 X 4	2 X 6	01-03-08 01-03-08	01-02-00 01-02-00	170.77 105.33		
	1	T18A HIP	6.00 0.00	35-11-00	09-02-08	2 X 4	2 X 6	01-03-08 00-00-00	01-02-00 01-04-08	160.38 109.67		
	2	T19A COMMON	6.00 0.00	35-11-00	10-03-00	2 X 4	2 X 6	01-03-08 00-00-00	01-02-00 01-04-08	345.82 209.34		
	3	T20A HALF HIP	6.00 0.00	35-11-00	08-04-08	2 X 4	2 X 6	01-03-08 00-00-00	01-02-00 08-04-08	570.87 345.00		
	1	T21A HALF HIP	6.00 0.00	35-11-00	09-08-08	2 X 4	2 X 6	01-03-08 00-00-00	01-02-00 09-08-08	203.45 121.67		
	3	T188 COMMON	6.00 0.00	28-02-00	08-02-08	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	330.48 206.01		
	1	T22 MONOPITCH	8.00 0.00	14-02-08	10-10-08	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 10-10-08	67.56 41.67		
	1	G22 MONOPITCH	8.00 0.00	14-02-08	10-10-08	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 10-10-08	77.96 48.67		
	1 3 Ply	T23 MONOPITCH	8.00 0.00	14-02-08	12-10-08	2 X 6	2 X 6	00-00-00 00-00-00	03-04-13 12-10-08	372.33 225.00		
	9	T90TCX MONOPITCH	3.00 0.00	10-04-00	03-05-09	2 X 6	2 X 4	00-09-08 00-00-00	00-08-04 03-05-09	404.91 272.97		
	2	G90 MONOPITCH	3.00 0.00	10-04-00	03-03-04	2 X 6	2 X 4	00-09-08 00-00-00	00-08-04 03-03-04	81.80 53.34		
	2	T92 COMMON	6.50 0.00	06-03-00	02-03-12	2 X 4	2 X 4	01-03-08 01-03-08	00-07-07 00-07-07	43.08 30.00		
	1	T92Z COMMON	6.50 0.00	06-03-00	02-03-12	2 X 4	2 X 4	01-03-08 01-03-08	00-07-07 00-07-07	21.54 15.00		
	1	P1 PIGGYBACK	6.00 0.00	20-06-11	02-06-08	2 X 4	2 X 4	00-00-00 00-00-00	00-00-00 02-08-00	69.01 42.67		
	1	P2 PIGGYBACK	6.00 0.00	20-06-11	03-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-00-00 04-00-00	69.87 44.00		
	3	J2 JACK-OPEN	8.00 0.00	05-10-08	05-03-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-03-13	53.49 32.01		

TAMARACK

Delivery Shiplist

DATE	09/07/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 285932 LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH 2 SUB-BUILDER:
MODEL: HIGHGROVE 2 ELEVATION: 1- (REV)- UNIT3

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	4	J93 MONOPITCH	4.00 / 0.00	06-05-00	02-05-10	2 X 4	2 X 4	01-03-08 / 00-00-00	00-03-15 / 02-05-10	72.64 / 46.68		
	3	J94 JACK-OPEN	8.00 / 0.00	02-03-04	02-03-13	2 X 4	2 X 4	01-03-08 / 00-00-00	00-09-10 / 02-03-13	24.27 / 18.00		

TOTAL # TRUSS= 44.00 TOTAL BFT OF ALL TRUSSES= 2200.37 BFT. TOTAL WEIGHT OF ALL TRUSSES= 3544.01 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
13	Hangers	LJS26DS	

TOTAL # ITEMS= 15.00

TAMARACK LUMBER INC.

Delivery Shiplist

DATE	09/08/17
SALES REP	Mario

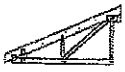
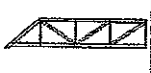
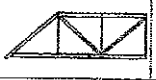
JOB TRACK: 43954 LAYOUT ID: 285298 D1 LOCATION:

BUILDER: GREENPARK - RUSSELL GARDENS PH2 SUB-BUILDER:

MODEL: HIGHGROVE 2 ELEVATION: EL:1- UNIT4

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	2	T16X COMMON	6.00 0.00	36-04-00	09-09-12	2 X 4	2 X 6	01-03-08 00-00-00	01-02-00 00-03-08	328.12 200.34		
	4	T17S ROOF	6.00 0.00	36-04-00	09-09-12	2 X 4	2 X 6	01-03-08 01-03-08	01-02-00 01-02-00	717.04 446.88		
	1	T18A HIP	6.00 0.00	35-11-00	09-02-08	2 X 4	2 X 6	01-03-08 00-00-00	01-02-00 01-04-08	180.38 109.67		
	2	T19A COMMON	6.00 0.00	35-11-00	10-03-00	2 X 4	2 X 6	01-03-08 00-00-00	01-02-00 01-04-08	345.82 209.34		
	3	T20A HALF HIP	6.00 0.00	35-11-00	08-04-08	2 X 4	2 X 6	01-03-08 00-00-00	01-02-00 08-04-08	570.87 345.00		
	1	T21A HALF HIP	6.00 0.00	35-11-00	09-08-08	2 X 4	2 X 6	01-03-08 00-00-00	01-02-00 09-08-08	203.45 121.67		
	1	T22 MONOPITCH	8.00 0.00	14-02-08	10-10-08	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 10-10-08	67.56 41.67		
	1	G22 MONOPITCH	8.00 0.00	14-02-08	10-10-08	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 10-10-08	77.96 48.67		
	1 3 Ply	T23 MONOPITCH	8.00 0.00	14-02-08	12-10-08	2 X 6	2 X 6	00-00-00 00-00-00	03-04-13 12-10-08	372.33 225.00		
	10	T90TCX MONOPITCH	3.00 0.00	10-04-00	03-05-09	2 X 6	2 X 4	00-09-08 00-00-00	00-08-04 03-05-09	449.90 303.30		
	1	G90 MONOPITCH	3.00 0.00	10-04-00	03-03-04	2 X 6	2 X 4	00-09-08 00-00-00	00-08-04 03-03-04	40.90 26.67		
	2	T92 COMMON	6.50 0.00	06-03-00	02-03-12	2 X 4	2 X 4	01-03-08 01-03-08	00-07-07 00-07-07	43.08 30.00		
	1	T92Z COMMON	6.50 0.00	06-03-00	02-03-12	2 X 4	2 X 4	01-03-08 01-03-08	00-07-07 00-07-07	21.54 15.00		
	1	P1 PIGGYBACK	6.00 0.00	20-06-11	02-06-08	2 X 4	2 X 4	00-00-00 00-00-00	00-00-00 02-08-00	69.01 42.67		
	1	P2 PIGGYBACK	6.00 0.00	20-06-11	03-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-00-00 04-00-00	69.87 44.00		
	4	J93 MONOPITCH	4.00 0.00	06-05-00	02-05-10	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-05-10	72.64 46.68		
	3	J94 JACK-OPEN	8.00 0.00	02-03-04	02-03-13	2 X 4	2 X 4	01-03-08 00-00-00	00-09-10 02-03-13	24.27 18.00		

TOTAL # TRUSS= 41.00 TOTAL BFT OF ALL TRUSSES= 2274.36 BFT. TOTAL WEIGHT OF ALL TRUSSES= 3654.74 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16

Delivery Shiplist

DATE	09/08/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 285298 01 LOCATION:

BUILDER: GREENPARK - RUSSELL GARDENS PH2 SUB-BUILDER:

MODEL: HIGHGROVE 2 ELEVATION: EL:1- UNIT4

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
7	Hangers	LJS26DS	

TOTAL # ITEMS= 7.00

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

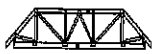
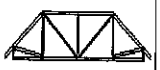
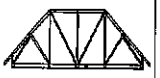
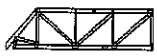
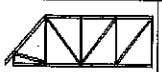
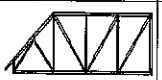
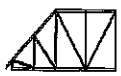
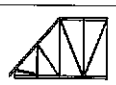
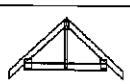
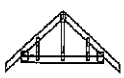
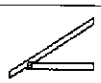
Delivery Shiplist

DATE	04/17/18
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 285934 LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH 2 SUB-BUILDER:
MODEL: HIGHGROVE 5E ELEVATION: 1 (REV)-UNIT5

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T1A COMMON	8.00 0.00	24-03-08	09-06-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	102.41 64.00		
	1	T2 HIP GIRDER	8.00 0.00	24-06-00	04-01-04	2 X 6	2 X 6	01-03-08 01-01-00	01-04-13 01-04-13	145.29 89.67		
	1	T3 HIP	8.00 0.00	24-06-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	97.09 60.83		
	1	T4 HIP	8.00 0.00	24-06-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	105.79 67.00		
	1	T5A HIP	8.00 0.00	24-03-08	07-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	104.32 67.00		
	1	T6A HIP	8.00 0.00	24-03-08	08-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	108.82 69.50		
	1	T7A HIP	8.00 0.00	24-03-08	09-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	114.67 72.50		
	1	T8 HALF HIP	8.00 0.00	24-03-08	04-01-04	2 X 6	2 X 6	01-03-08 00-00-00	01-04-13 04-01-04	138.05 86.67		
	2	T9 HALF HIP	8.00 0.00	24-03-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-01-04	197.10 123.66		
	2	T10 HALF HIP	8.00 0.00	24-03-08	06-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 06-01-04	212.66 133.34		
	2	T11 HALF HIP	8.00 0.00	24-03-08	07-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 07-01-04	225.56 142.68		
	2	T12 HALF HIP	8.00 0.00	24-03-08	08-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 08-01-04	229.86 143.66		
	3	T13 HALF HIP	8.00 0.00	24-03-08	09-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 09-01-04	360.48 222.99		
	6	T14 COMMON	8.00 0.00	14-04-00	06-02-02	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	370.56 234.00		
	3	T91 COMMON	6.50 0.00	06-06-00	02-04-09	2 X 4	2 X 4	01-03-08 01-03-08	00-07-07 00-07-07	66.60 45.00		
	1	G91 COMMON	6.50 0.00	06-06-00	02-04-09	2 X 4	2 X 4	01-03-08 01-03-08	00-07-07 00-07-07	23.39 16.00		
	16	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	268.64 170.72		
	1	J90 MONOPITCH	4.50 0.00	05-05-00	02-05-00	2 X 4	2 X 4	01-03-08 00-00-00	00-04-10 02-05-00	14.47 9.33		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	04/17/18
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 285934 LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH 2 SUB-BUILDER:
MODEL: HIGHGROVE 5E ELEVATION: 1 (REV)-UNIT5

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	3	J92 MONOPITCH	4.50 0.00	04-03-00	01-11-12	2 X 4	2 X 4	01-03-08 00-00-00	00-04-10 01-11-12	35.16 24.00		

TOTAL # TRUSS= **49.00** TOTAL BFT OF ALL TRUSSES= **1842.55** BFT. TOTAL WEIGHT OF ALL TRUSSES= **2920.92** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	HGUS26-2	
3	Hangers	LJS26DS	

TOTAL # ITEMS= **4.00**

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285994	T1A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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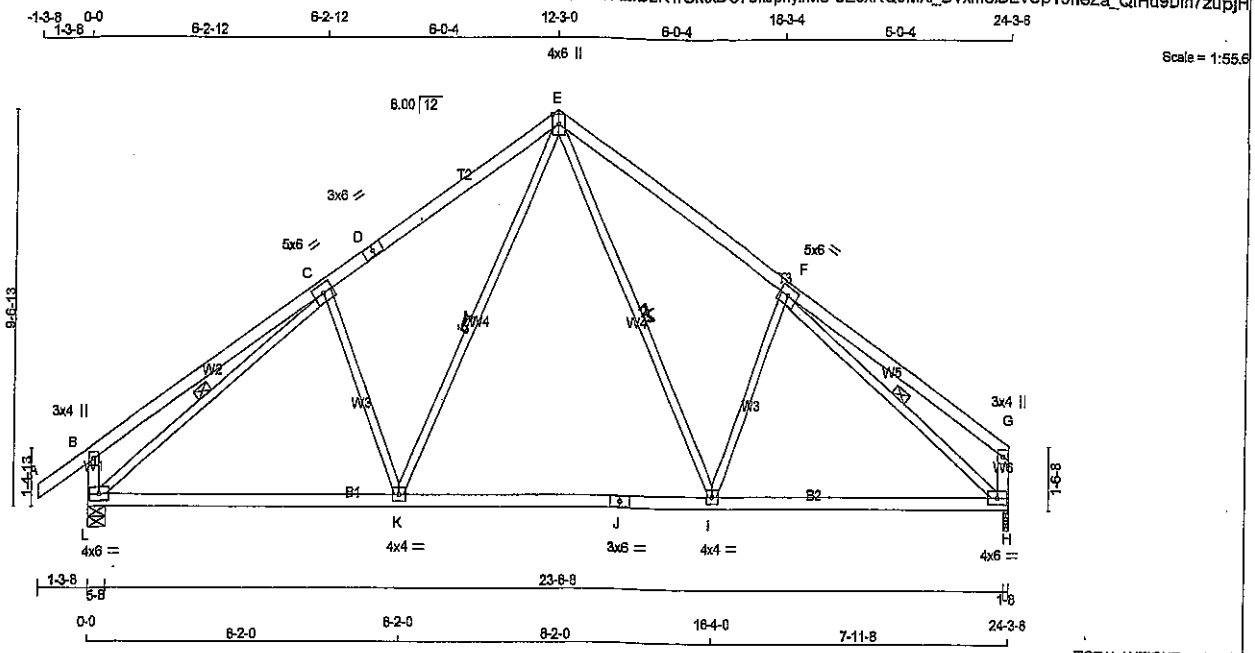

TOTAL WEIGHT = 102 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER		DESGR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	8.0		
D	TS-t	MT20	3.0	6.0		
E	TTWW+p	MT20	4.0	8.0	Edge	
F	TMWW-t	MT20	5.0	6.0		
G	TMV+p	MT20	3.0	4.0		
H	BMWW1-t	MT20	4.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1479	0	1479	0	0	5-8	5-8
H	1363	0	1363	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	OEAD	SOIL
L	1198	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0
H	1119	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.86 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-L, F-H. E-K E-I

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00
B-C	0 / 34	-84.3	-84.3	0.52 (1)	10.00
C-D	-1409 / 0	-84.3	-84.3	0.45 (1)	4.96
D-E	-1409 / 0	-84.3	-84.3	0.45 (1)	4.96
E-F	-1388 / 0	-84.3	-84.3	0.44 (1)	5.01
F-G	0 / 34	-84.3	-84.3	0.50 (1)	10.00
L-B	-311 / 0	0.0	0.0	0.03 (1)	7.81
H-G	-186 / 0	0.0	0.0	0.02 (1)	7.81
L-K	0 / 1275	-28.0	-28.0	0.58 (2)	10.00
K-J	0 / 889	-28.0	-28.0	0.54 (2)	10.00
J-I	0 / 889	-28.0	-28.0	0.54 (2)	10.00
I-H	0 / 1245	-28.0	-28.0	0.56 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-K	-352 / 44	0.19 (1)
K-E	0 / 632	0.14 (1)
E-I	0 / 589	0.13 (1)
I-F	-316 / 56	0.17 (1)
L-C	-1669 / 0	0.71 (1)
F-H	-1653 / 0	0.67 (1)

LICENSED PROFESSIONAL ENGINEER
1-16-18
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.29")

CSI: TC=0.52/1.00 (B-C:1) , BC=0.58/1.00 (K-L:2) , WB=0.71/1.00 (C-L:1) , SSI=0.21/1.00 (C-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (L) (INPUT = 0.90)
JSI METAL= 0.43 (C) (INPUT = 1.00)

DWG NO. TAM3984-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285929	T2	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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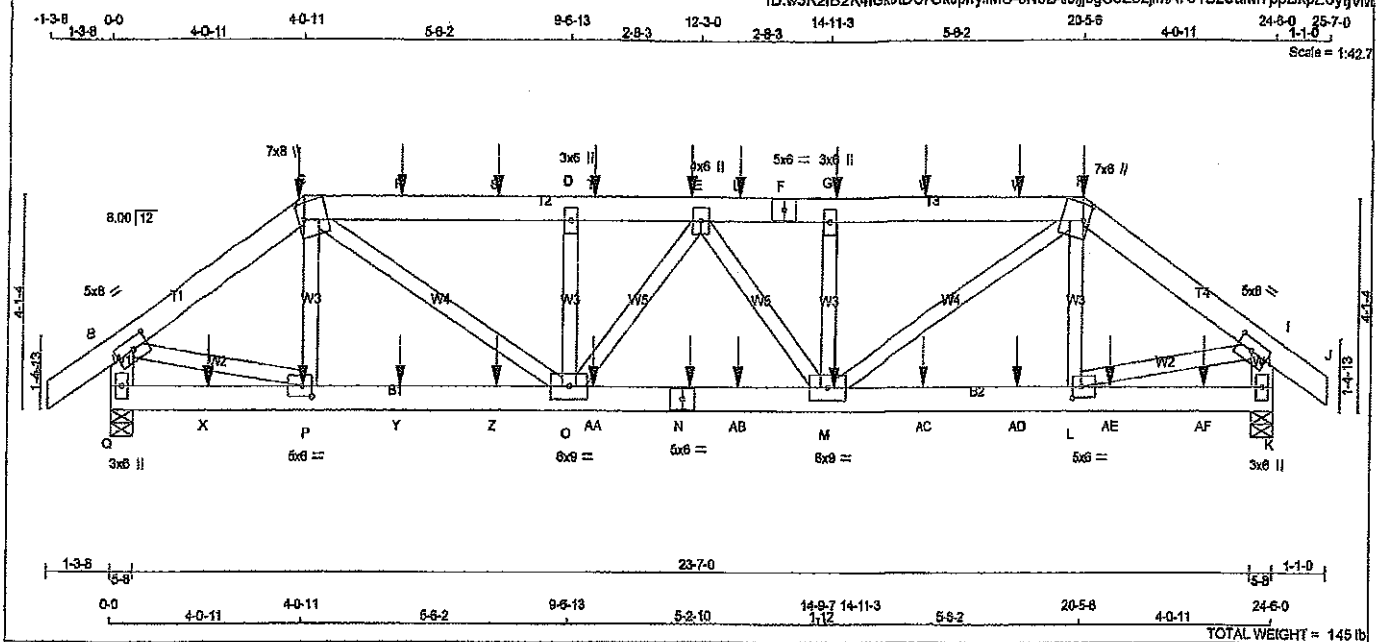

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
Q - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
Q - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	5.0	6.0	2.50	3.75
C	TTWW+m	MT20	7.0	8.0	3.75	1.50
D	TMW+w	MT20	3.0	6.0		
E	TMWW+t	MT20	4.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMW+w	MT20	3.0	6.0		
H	TTWW+m	MT20	7.0	6.0	3.75	1.50
I	TMW-t	MT20	5.0	6.0	2.50	3.75
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.25
M	BMWWW-t	MT20	6.0	9.0		
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	6.0	9.0		
P	BMWW-t	MT20	5.0	6.0	2.50	2.25
Q	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 349.6 lbs FACTORED DOWN AT 4-0-11, 244.5 lbs FACTORED DOWN AT 20-5-5, 101.4 lbs FACTORED DOWN AT 6-0-12, 101.4 lbs FACTORED DOWN AT 8-0-12, 101.4 lbs FACTORED DOWN AT 10-0-12, 101.4 lbs FACTORED DOWN AT 12-0-12, 101.4 lbs FACTORED DOWN AT 13-0-12, 101.4 lbs FACTORED DOWN AT 15-0-12, AND 101.4 lbs FACTORED DOWN AT 17-0-12, AND 101.4 lbs FACTORED DOWN AT 19-0-12 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 2-0-12, 69.9 lbs FACTORED DOWN AT 4-0-12, 69.9 lbs FACTORED DOWN AT 6-0-12, 69.9 lbs FACTORED DOWN AT 8-0-12, 69.9 lbs FACTORED DOWN AT 10-0-12, 69.9 lbs FACTORED DOWN AT 12-0-12, 69.9 lbs FACTORED DOWN AT 13-0-12, 69.9 lbs FACTORED DOWN AT 15-0-12, 69.9 lbs FACTORED DOWN AT 17-0-12, 69.9 lbs FACTORED DOWN AT 19-0-12, AND 69.9 lbs FACTORED DOWN AT 21-0-12, AND 69.9 lbs FACTORED DOWN AT 23-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	2456	0	2456	0	0	5-8	5-8
K	2401	0	2401	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1996	1139 / 0	433 / 0	0 / 0	0 / 0	425 / 0	0 / 0
K	1966	1100 / 0	441 / 0	0 / 0	0 / 0	424 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.94 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 33	-84.3	-84.3	0.07 (1)	10.00	P-C	-251 / 184	0.04 (1)
B-C	-2866 / 0	-84.3	-84.3	0.18 (1)	4.77	C-O	0 / 1691	0.33 (1)
C-R	-3948 / 0	-84.3	-84.3	0.40 (1)	3.94	O-D	-696 / 0	0.12 (1)
R-S	-3948 / 0	-84.3	-84.3	0.40 (1)	3.94	M-G	-718 / 0	0.12 (1)
S-D	-3948 / 0	-84.3	-84.3	0.40 (1)	3.94	M-H	0 / 1944	0.34 (1)
D-T	-3949 / 0	-84.3	-84.3	0.39 (1)	4.05	L-H	-235 / 190	0.04 (1)
T-E	-3949 / 0	-84.3	-84.3	0.39 (1)	4.05	B-P	0 / 2432	0.43 (1)
E-U	-3940 / 0	-84.3	-84.3	0.29 (1)	4.06	L-I	0 / 2378	0.42 (1)
U-F	-3940 / 0	-84.3	-84.3	0.29 (1)	4.06	O-E	-99 / 0	0.02 (1)
F-G	-3940 / 0	-84.3	-84.3	0.29 (1)	4.06	E-M	-113 / 0	0.03 (1)
G-V	-3940 / 0	-84.3	-84.3	0.39 (1)	3.96			
V-W	-3940 / 0	-84.3	-84.3	0.39 (1)	3.96			
W-H	-3940 / 0	-84.3	-84.3	0.39 (1)	3.96			
H-I	-2802 / 0	-84.3	-84.3	0.17 (1)	4.82			
I-J	0 / 28	-84.3	-84.3	0.05 (1)	10.00			
Q-B	-2397 / 0	0.0	0.0	0.17 (1)	8.61			
K-I	-2332 / 0	0.0	0.0	0.16 (1)	8.69			
Q-X	0 / 0	-28.0	-28.0	0.12 (3)	10.00			
X-P	0 / 0	-28.0	-28.0	0.12 (3)	10.00			
P-Y	0 / 2368	-28.0	-28.0	0.38 (1)	10.00			
Y-Z	0 / 2368	-28.0	-28.0	0.38 (1)	10.00			
Z-O	0 / 2368	-28.0	-28.0	0.38 (1)	10.00			
O-AA	0 / 4008	-28.0	-28.0	0.63 (1)	10.00			
AA-N	0 / 4008	-28.0	-28.0	0.63 (1)	10.00			
N-AB	0 / 4008	-28.0	-28.0	0.63 (1)	10.00			
AB-M	0 / 4008	-28.0	-28.0	0.63 (1)	10.00			
M-AC	0 / 2316	-28.0	-28.0	0.37 (1)	10.00			
AC-AD	0 / 2316	-28.0	-28.0	0.37 (1)	10.00			
AD-L	0 / 2316	-28.0	-28.0	0.37 (1)	10.00			
L-AE	0 / 0	-28.0	-28.0	0.14 (3)	10.00			
AE-AF	0 / 0	-28.0	-28.0	0.14 (3)	10.00			
AF-K	0 / 0	-28.0	-28.0	0.14 (3)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	4-0-11	-23	-25	—	FRONT	VERT	DEAD
C	4-0-11	-105	-105	—	FRONT	VERT	TOTAL
C	4-0-11	-222	-222	—	FRONT	VERT	SNOW
E	12-0-12	-101	-101	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.62")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.62")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.40 (C-D:1) , BC=0.63 (M-O:1) , WB=0.43 (B-P:1) , SSI=0.26 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.71 (N) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO . TAM 43482-17

STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43854	DRWG NO.
285929	T2	1	1	TRUSS DESC.	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
G	15-0-12	-101	-101	--	FRONT	VERT	TOTAL
H	20-5-5	-23	-25	--	FRONT	VERT	DEAD
H	20-5-5	-222	-222	--	FRONT	VERT	SNOW
M	15-0-12	-40	-70	--	FRONT	VERT	TOTAL
N	12-0-12	-40	-70	--	FRONT	VERT	TOTAL
P	4-0-12	-40	-70	--	FRONT	VERT	TOTAL
R	6-0-12	-101	-101	--	FRONT	VERT	TOTAL
S	8-0-12	-101	-101	--	FRONT	VERT	TOTAL
T	10-0-12	-101	-101	--	FRONT	VERT	TOTAL
U	13-0-12	-101	-101	--	FRONT	VERT	TOTAL
V	17-0-12	-101	-101	--	FRONT	VERT	TOTAL
W	19-0-12	-101	-101	--	FRONT	VERT	TOTAL
X	2-0-12	-40	-70	--	FRONT	VERT	TOTAL
Y	6-0-12	-40	-70	--	FRONT	VERT	TOTAL
Z	8-0-12	-40	-70	--	FRONT	VERT	TOTAL
AA	10-0-12	-40	-70	--	FRONT	VERT	TOTAL
AB	13-0-12	-40	-70	--	FRONT	VERT	TOTAL
AC	17-0-12	-40	-70	--	FRONT	VERT	TOTAL
AD	19-0-12	-40	-70	--	FRONT	VERT	TOTAL
AE	21-0-12	-40	-70	--	FRONT	VERT	TOTAL
AF	23-0-12	-40	-70	--	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
9/11/17
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 45482-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	ORWG NO.
285929	T3	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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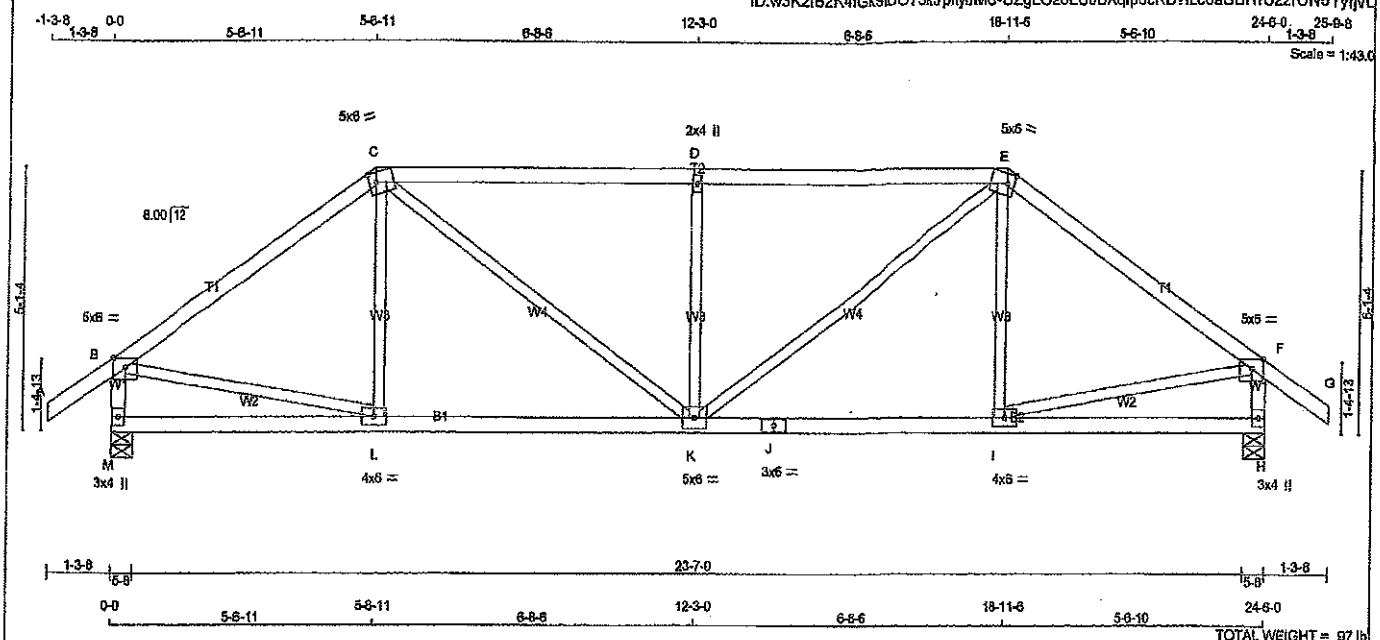


LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW-p	MT20	5.0	8.0	Edge	
C	TTWW-m	MT20	5.0	6.0	2.25	1.75
D	TMW+w	MT20	2.0	4.0		
E	TTWW-m	MT20	5.0	6.0	2.25	1.75
F	TMW-p	MT20	5.0	8.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	4.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	5.0	8.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1491	0	1491	0	0	5-8	5-8
H	1491	0	1491	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1208	697 / 0	257 / 0	0 / 0	0 / 0	253 / 0	0 / 0
H	1208	697 / 0	257 / 0	0 / 0	0 / 0	253 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.85 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX. (PLF) CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00
B-C	-1511 / 0	-84.3	-84.3	0.55 (1)	4.67
C-D	-1662 / 0	-84.3	-84.3	0.80 (1)	3.85
D-E	-1662 / 0	-84.3	-84.3	0.80 (1)	3.85
E-F	-1511 / 0	-84.3	-84.3	0.55 (1)	4.67
F-G	0 / 32	-84.3	-84.3	0.11 (1)	10.00
M-B	-1430 / 0	0.0	0.0	0.15 (1)	6.83
H-F	-1430 / 0	0.0	0.0	0.15 (1)	6.83
M-L	0 / 0	-28.0	-28.0	0.25 (3)	10.00
L-K	0 / 1255	-28.0	-28.0	0.40 (2)	10.00
K-J	0 / 1255	-28.0	-28.0	0.40 (2)	10.00
J-I	0 / 1255	-28.0	-28.0	0.49 (2)	10.00
I-H	0 / 0	-28.0	-28.0	0.26 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L-C	-47 / 210	0.05 (3)
C-K	0 / 747	0.17 (1)
K-D	-692 / 0	0.27 (1)
K-E	0 / 747	0.17 (1)
I-E	-47 / 210	0.05 (3)
B-L	0 / 1278	0.29 (1)
I-F	0 / 1278	0.29 (1)

DWG NO. TAM45483-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.82")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.82")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.80 (C-D:1) , BC=0.40 (K-L:2) , WB=0.29 (B-L:1) , SSI=0.27 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

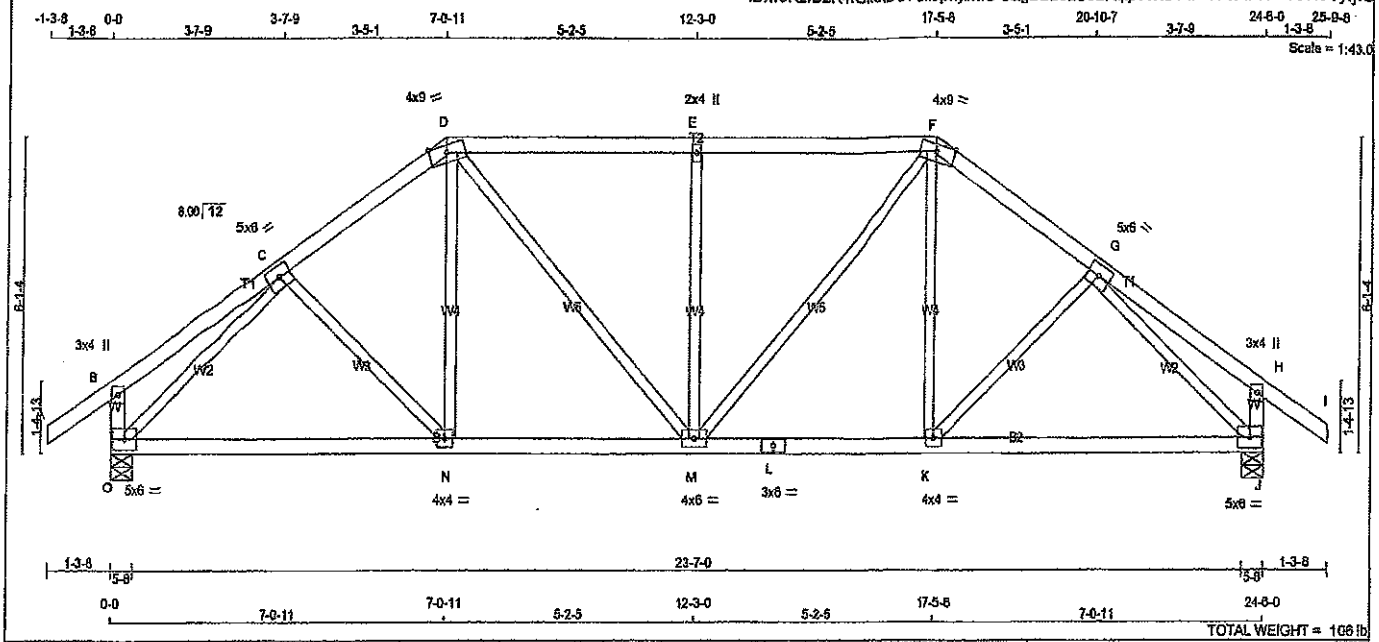
PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.64 (L) (INPUT = 0.90)
JSI METAL= 0.37 (J) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285929	T4	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington



TOTAL WEIGHT = 106 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW-m	MT20	4.0	9.0	Edge	
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	4.0	9.0	Edge	
G	TMWW-t	MT20	5.0	6.0		
H	TMV+p	MT20	3.0	4.0		
J	BMWW1-t	MT20	5.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMWW1-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1491	0	1491	0	0	5-8	5-8
J	1491	0	1491	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN. COMPONENT REACTIONS SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
O	1208	697 / 0	257 / 0	0 / 0	0 / 0	253 / 0	0 / 0
J	1208	697 / 0	257 / 0	0 / 0	0 / 0	253 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.02 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

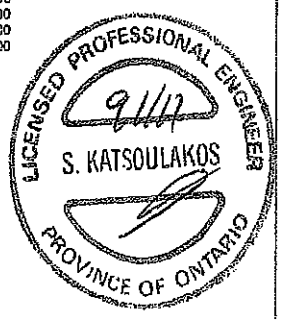
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC (FROM	TO) (PLF)	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00
B-C	0 / 17	-84.3	-84.3	0.15 (1)	10.00
C-D	-1476 / 0	-84.3	-84.3	0.14 (1)	5.27
D-E	-1498 / 0	-84.3	-84.3	0.32 (1)	5.02
E-F	-1498 / 0	-84.3	-84.3	0.32 (1)	5.02
F-G	-1476 / 0	-84.3	-84.3	0.14 (1)	5.27
G-H	0 / 17	-84.3	-84.3	0.15 (1)	10.00
H-I	0 / 32	-84.3	-84.3	0.11 (1)	10.00
O-B	-233 / 0	0.0	0.0	0.02 (1)	7.81
J-H	-233 / 0	0.0	0.0	0.02 (1)	7.81
O-N	0 / 1208	-26.0	-26.0	0.45 (2)	10.00
N-M	0 / 1216	-26.0	-26.0	0.47 (2)	10.00
M-L	0 / 1216	-26.0	-26.0	0.47 (2)	10.00
L-K	0 / 1216	-26.0	-26.0	0.47 (2)	10.00
K-J	0 / 1208	-26.0	-26.0	0.45 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-N	0 / 119	0.03 (3)
N-D	0 / 304	0.07 (2)
D-M	0 / 424	0.10 (1)
M-E	-533 / 0	0.31 (1)
M-F	0 / 424	0.10 (1)
K-F	0 / 304	0.07 (2)
K-G	0 / 119	0.03 (3)
O-C	-1686 / 0	0.65 (1)
G-J	-1686 / 0	0.65 (1)



DWG NO. TAM4548417
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.82")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.82")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.32 (D-E:1) , BC=0.47 (M-N:2) , WB=0.65 (C-O:1) , SSI=0.21 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1657	822	2284	1655

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (F) (INPUT = 0.90)
JSI METAL= 0.41 (C) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285994	T5A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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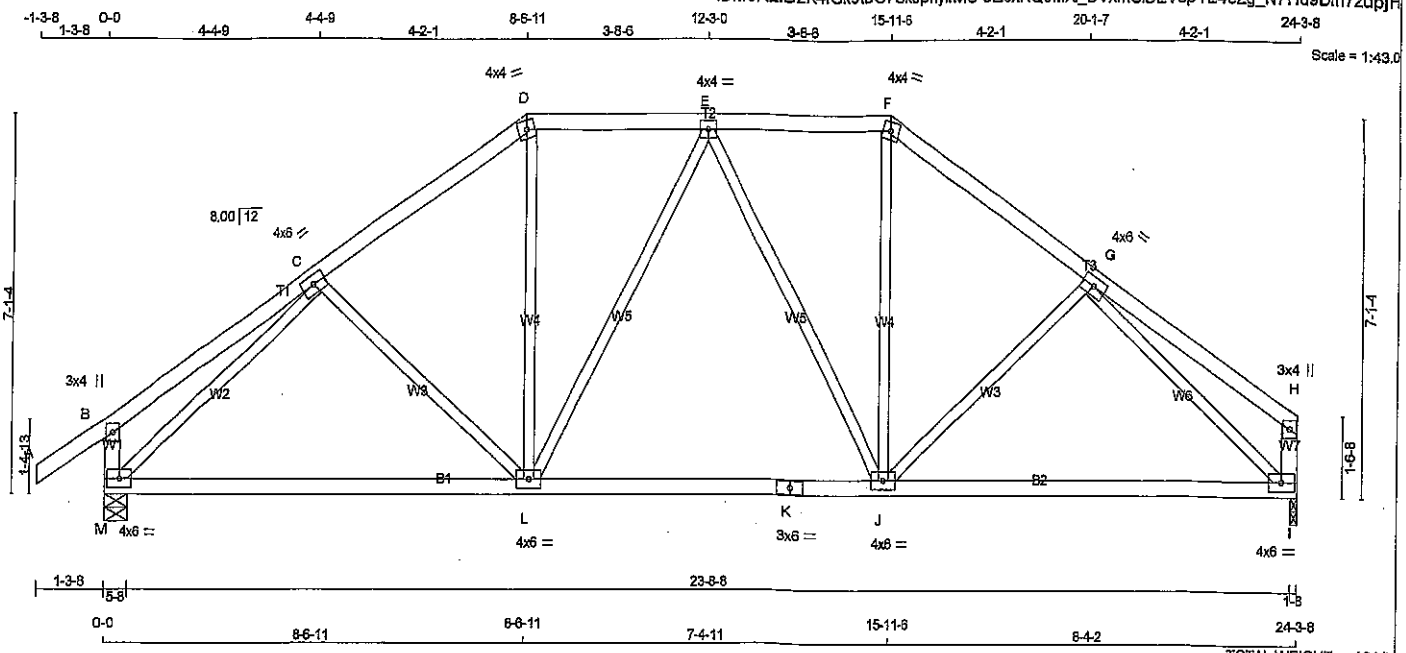

TOTAL WEIGHT = 104 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER		DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	4.0		
G	TMWW-t	MT20	4.0	6.0		
H	TMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	6.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	4.0	6.0		
M	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
M	1479	0	1479	0	0	5-8	5-8
I	1363	0	1363	0	0	1-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1198	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0
I	1119	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.32 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.3	-84.3	0.11 (1)		10.00
B- C	0 / 22	-84.3	-84.3	0.24 (1)		10.00
C- D	-1368 / 0	-84.3	-84.3	0.21 (1)		5.32
D- E	-1142 / 0	-84.3	-84.3	0.15 (1)		5.79
E- F	-1128 / 0	-84.3	-84.3	0.15 (1)		5.82
F- G	-1371 / 0	-84.3	-84.3	0.20 (1)		5.35
G- H	0 / 22	-84.3	-84.3	0.23 (1)		10.00
M- B	-255 / 0	0.0	0.0	0.03 (1)		7.81
I- H	-131 / 0	0.0	0.0	0.01 (1)		7.81
M- L	0 / 1222	-28.0	-28.0	0.57 (2)		10.00
L- K	0 / 1233	-28.0	-28.0	0.58 (2)		10.00
K- J	0 / 1233	-28.0	-28.0	0.58 (2)		10.00
J- I	0 / 1173	-26.0	-28.0	0.54 (2)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- L	-117 / 85	0.07 (1)
L- D	0 / 495	0.11 (1)
L- E	-195 / 0	0.24 (1)
E- J	-225 / 0	0.27 (1)
J- F	0 / 484	0.11 (1)
J- G	-70 / 102	0.04 (1)
M- C	-1660 / 0	0.93 (1)
G- I	-1632 / 0	0.86 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.23")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 761 (0.36")

CSI: TC=0.24/1.00 (B-C:1) , BC=0.58/1.00 (J-L:2) , WB=0.93/1.00 (C-M:1) , SSI=0.19/1.00 (L-M:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (M) (INPUT = 0.90)
JSI METAL= 0.40 (C) (INPUT = 1.00)

DWG NO. TAM 3985-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285994	T6A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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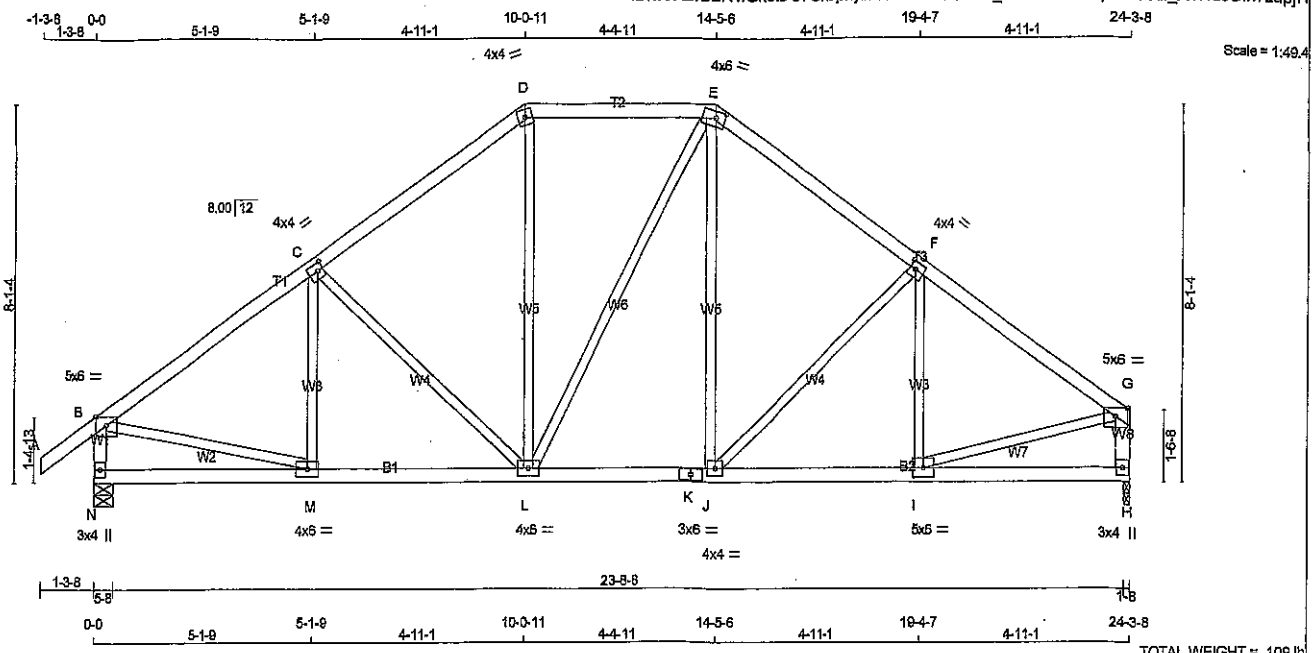


TOTAL WEIGHT = 109 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTW-m	MT20	4.0	4.0		
E	TTWW-m	MT20	4.0	6.0	1.75	2.50
F	TMWW-t	MT20	4.0	4.0	2.00	1.50
G	TMVW-p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	5.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	4.0	6.0		
M	BMWW-t	MT20	4.0	6.0		
N	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
N	1479	0	1479	0	0	5-8	5-8
H	1363	0	1363	0	0	1-8	1-8

UNFACTORED REACTIONS

	1ST LOASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1198	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0
H	1119	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.3	-84.3	0.11 (1)		10.00
B-C	-1531 / 0	-84.3	-84.3	0.31 (1)		5.00
C-D	-1271 / 0	-84.3	-84.3	0.29 (1)		5.39
D-E	-1037 / 0	-84.3	-84.3	0.22 (1)		5.91
E-F	-1262 / 0	-84.3	-84.3	0.28 (1)		5.42
F-G	-1483 / 0	-84.3	-84.3	0.29 (1)		5.08
N-B	-1420 / 0	0.0	0.0	0.15 (1)		6.84
H-G	-1307 / 0	0.0	0.0	0.14 (1)		7.06
N-M	0 / 0	-28.0	-28.0	0.18 (3)		10.00
M-L	0 / 1296	-28.0	-28.0	0.32 (2)		10.00
L-K	0 / 1030	-28.0	-28.0	0.23 (2)		10.00
K-J	0 / 1030	-28.0	-28.0	0.23 (2)		10.00
J-I	0 / 1257	-28.0	-28.0	0.31 (2)		10.00
I-H	0 / 0	-28.0	-28.0	0.17 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M-C	-115 / 147	0.04 (1)
C-L	-354 / 0	0.30 (1)
L-D	0 / 374	0.08 (2)
L-E	0 / 15	0.00 (1)
J-E	0 / 360	0.08 (2)
J-F	-310 / 0	0.26 (1)
I-F	-156 / 122	0.05 (1)
B-M	0 / 1325	0.30 (1)
I-G	0 / 1294	0.29 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	8.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	48.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(65 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 989 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.31/1.00 (B-C:1) , BC=0.32/1.00 (L-M:2) , WB=0.30/1.00 (C-L:1) , SSI=0.17/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

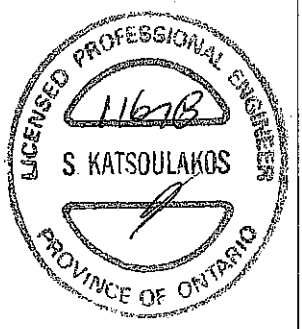
NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (M) (INPUT = 0.80)
JSI METAL= 0.32 (M) (INPUT = 1.00)



DWG NO. TAM 3986-13
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285994	T7A	1	1	TRUSS DESC.	

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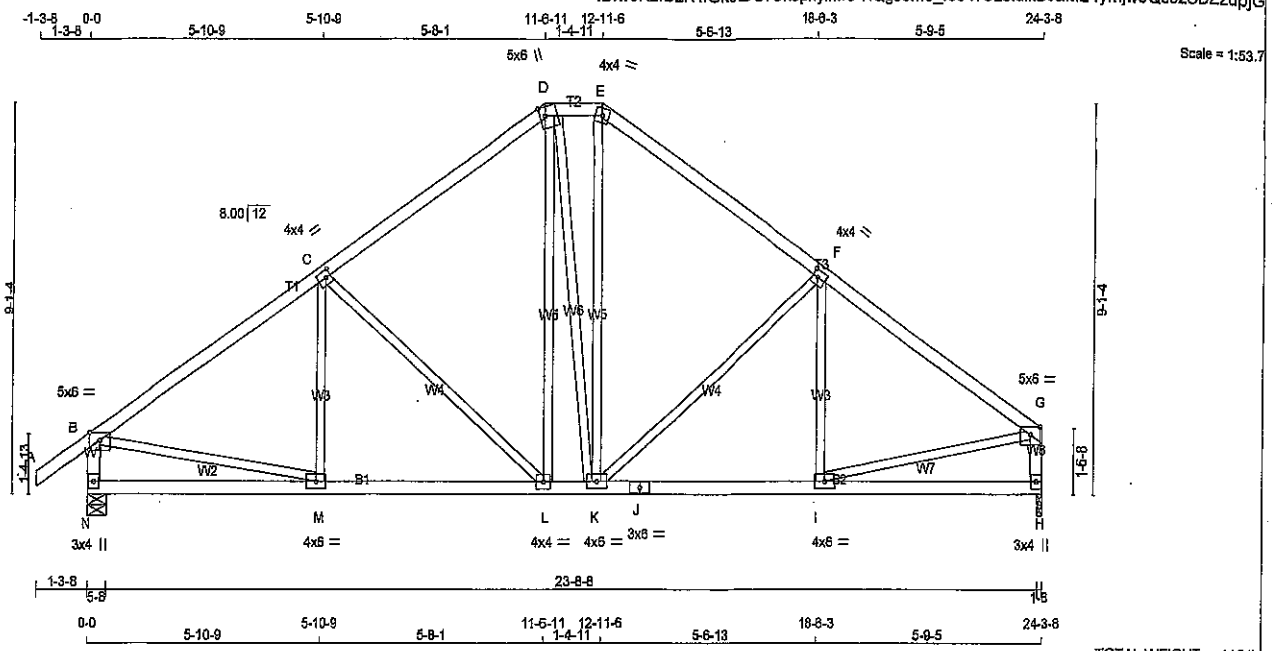

TOTAL WEIGHT = 115 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESGR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.50	1.50
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0	2.00	1.50
G	TMWW-p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMWW-t	MT20	4.0	6.0		
N	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
N	1479	0	1479	0	0	5-8	5-8
H	1363	0	1363	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1198	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0
H	1119	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.86 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00
B-C	-1533 / 0	-84.3	-84.3	0.42 (1)	4.86
C-D	-1163 / 0	-84.3	-84.3	0.38 (1)	5.44
D-E	-945 / 0	-84.3	-84.3	0.03 (1)	6.25
E-F	-1164 / 0	-84.3	-84.3	0.37 (1)	5.46
F-G	-1491 / 0	-84.3	-84.3	0.39 (1)	4.94
N-B	-1413 / 0	0.0	0.0	0.15 (1)	6.86
H-G	-1299 / 0	0.0	0.0	0.14 (1)	7.08
N-M	0 / 0	-28.0	-28.0	0.25 (3)	10.00
M-L	0 / 1302	-28.0	-28.0	0.40 (2)	10.00
L-K	0 / 943	-28.0	-28.0	0.26 (2)	10.00
K-J	0 / 1268	-28.0	-28.0	0.37 (2)	10.00
J-I	0 / 1268	-28.0	-28.0	0.37 (2)	10.00
I-H	0 / 0	-28.0	-28.0	0.23 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M-C	-50 / 214	0.05 (3)
C-L	-461 / 0	0.57 (1)
L-D	0 / 377	0.08 (1)
D-K	0 / 17	0.00 (2)
K-E	0 / 390	0.09 (1)
K-F	-433 / 0	0.51 (1)
I-F	-94 / 185	0.04 (3)
B-M	0 / 1324	0.30 (1)
I-G	0 / 1296	0.29 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.42/1.00 (B-C:1) , BC=0.40/1.00 (L-M:2) , WB=0.57/1.00 (C-L:1) , SSI=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (I) (INPUT = 0.90)
JSI METAL= 0.33 (M) (INPUT = 1.00)

DWG NO. TAM 3987 -12
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285934	T8	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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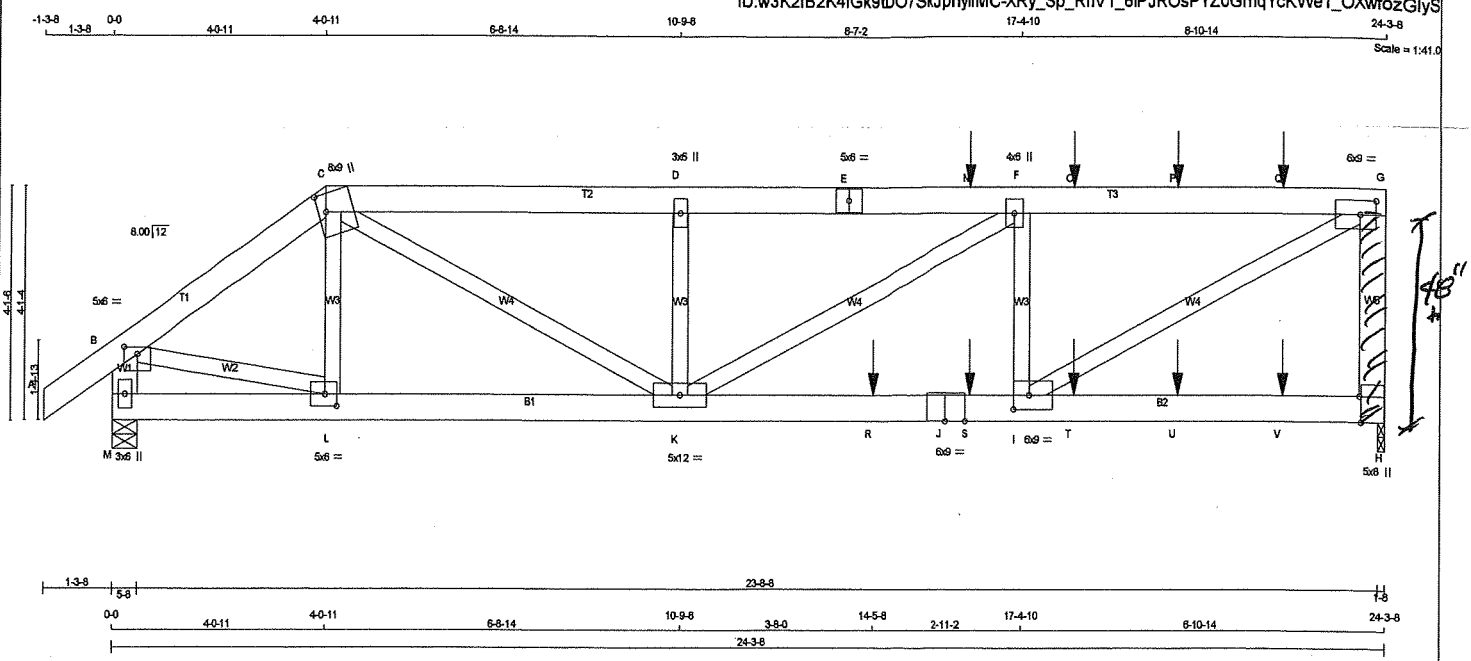

TOTAL WEIGHT = 138 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - G	2x6	DRY	No.2	SPF
H - G	2x6	DRY	No.2	SPF
M - B	2x6	DRY	No.2	SPF
M - J	2x6	DRY	2100F 1.8E	SPF
J - H	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TTWW+m	MT20	8.0	9.0	3.75	1.75
D	TMW+w	MT20	3.0	6.0		
E	TS-t	MT20	5.0	6.0		
F	TMWW+t	MT20	4.0	6.0		
G	TMVW-t	MT20	6.0	9.0	3.00	3.50
H	BMV1+p	MT20	5.0	8.0	Edge	0.25
I	BMWW-t	MT20	6.0	9.0	3.00	3.50
J	BS-t	MT20	6.0	9.0		
K	BMWWW-t	MT20	5.0	12.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.75
M	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1)

+ 48" VERTICAL SCAB

SCAB JOINT # H &# WITH 2 X 6 #2 SPF LONG 1 SIDE(S) USING 2 ROW(S) 3½" ARDOX SPIRAL/~~COMMON WIRE~~ NAILS AT 3" O/C STAGGERED (8 NAILS/SCAB). REQUIRED FOR 1½" BRG PROVISION.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	2719	0	2719	0	0	1-8	1-8
M	2212	0	2212	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	2211	1262 / 0	480 / 0	0 / 0	0 / 0	468 / 0	0 / 0
M	1781	1046 / 0	367 / 0	0 / 0	0 / 0	368 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.50 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 33	-84.3	-84.3	0.07 (1)	10.00	L- C	-437 / 17	0.07 (1)
B- C	-2504 / 0	-84.3	-84.3	0.17 (1)	5.05	B- L	0 / 2124	0.38 (1)
C- D	-4665 / 0	-84.3	-84.3	0.45 (1)	3.64	I- G	0 / 5007	0.89 (1)
D- E	-4665 / 0	-84.3	-84.3	0.55 (1)	3.50	C- K	0 / 2958	0.52 (1)
E- N	-4665 / 0	-84.3	-84.3	0.55 (1)	3.50	I- F	-1063 / 0	0.18 (1)
N- F	-4665 / 0	-84.3	-84.3	0.55 (1)	3.50	K- D	-564 / 0	0.10 (1)
F- O	-4428 / 0	-84.3	-84.3	0.61 (1)	3.50	K- F	0 / 272	0.05 (1)
O- P	-4428 / 0	-84.3	-84.3	0.61 (1)	3.50			
P- Q	-4428 / 0	-84.3	-84.3	0.61 (1)	3.50			
Q- G	-4428 / 0	-84.3	-84.3	0.61 (1)	3.50			
H- G	-2691 / 0	0.0	0.0	0.39 (1)	6.32			
M- B	-2131 / 0	0.0	0.0	0.15 (1)	6.93			
M- L	0 / 0	-28.0	-28.0	0.05 (1)	10.00			
L- K	0 / 2060	-28.0	-28.0	0.39 (1)	10.00			
K- R	0 / 4428	-28.0	-28.0	0.98 (1)	10.00			
R- J	0 / 4428	-28.0	-28.0	0.98 (1)	10.00			
J- S	0 / 4428	-28.0	-28.0	0.98 (1)	10.00			
S- I	0 / 4428	-28.0	-28.0	0.98 (1)	10.00			
I- T	0 / 0	-28.0	-28.0	0.33 (1)	10.00			
T- U	0 / 0	-28.0	-28.0	0.33 (1)	10.00			
U- V	0 / 0	-28.0	-28.0	0.33 (1)	10.00			
V- H	0 / 0	-28.0	-28.0	0.33 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
N	16-4-12	-69	-69	—	BACK	VERT	TOTAL
O	18-4-12	-69	-69	—	BACK	VERT	TOTAL
P	20-4-12	-69	-69	—	BACK	VERT	TOTAL
Q	22-4-12	-69	-69	—	BACK	VERT	TOTAL
R	14-5-8	-1197	-1197	—	BACK	VERT	TOTAL
S	16-4-12	-50	-50	—	BACK	VERT	TOTAL
T	18-4-12	-50	-50	—	BACK	VERT	TOTAL
U	20-4-12	-50	-50	—	BACK	VERT	TOTAL
V	22-4-12	-50	-50	—	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER 51518 S. KATSOULAKOS PROVINCE OF ONTARIO

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 968 (0.30")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 615 (0.47")

CSI: TC=0.61/1.00 (F-G:1) , BC=0.98/1.00 (I-K:1) , WB=0.89/1.00 (G-I:1) , SSI=0.66/1.00 (I-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (L) (INPUT = 0.90)
JSI METAL= 0.91 (J) (INPUT = 1.00)

CONTINUED ON PAGE 2

P6/4

DWG NO. TAM21154-18 RM
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285934	T8	1	1	TRUSS DESC.	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 101.4 lbs FACTORED DOWN AT 16-4-12, 101.4 lbs FACTORED DOWN AT 18-4-12, AND 101.4 lbs FACTORED DOWN AT 20-4-12, AND 101.4 lbs FACTORED DOWN AT 22-4-12 ON TOP CHORD, AND 1519.0 lbs FACTORED DOWN AT 14-5-8, 69.9 lbs FACTORED DOWN AT 16-4-12, 69.9 lbs FACTORED DOWN AT 18-4-12, AND 69.9 lbs FACTORED DOWN AT 20-4-12, AND 69.9 lbs FACTORED DOWN AT 22-4-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DWG NO. TAM 21154 -18 RH
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285994	T9	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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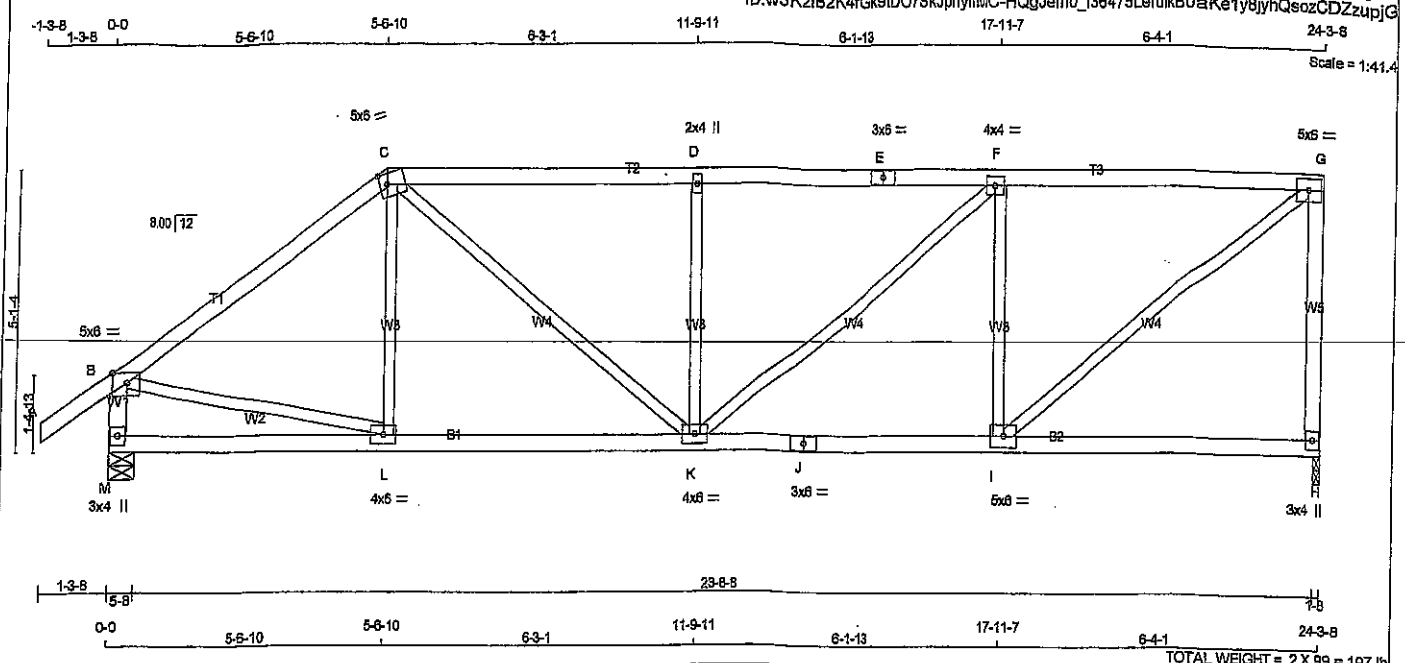

TOTAL WEIGHT = 2 X 99 = 197 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TTWW-m	MT20	5.0	6.0	2.25	1.75
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	5.0	6.0		
J	BG-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	1363	0	1353	0	0	1-8	1-8
M	1479	0	1479	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1119	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
M	1198	592 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.34 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00
B-C	-1493 / 0	-84.3	-84.3	0.55 (1)	4.89
C-D	-1797 / 0	-84.3	-84.3	0.55 (1)	4.37
D-E	-1797 / 0	-84.3	-84.3	0.55 (1)	4.34
E-F	-1797 / 0	-84.3	-84.3	0.56 (1)	4.34
F-G	-1416 / 0	-84.3	-84.3	0.53 (1)	4.81
H-G	-1292 / 0	0.0	0.0	0.57 (1)	7.10
M-B	-1417 / 0	0.0	0.0	0.15 (1)	6.85
M-L	0 / 0	-28.0	-28.0	0.24 (3)	10.00
L-K	0 / 1240	-28.0	-28.0	0.39 (2)	10.00
K-J	0 / 1416	-28.0	-28.0	0.44 (2)	10.00
J-I	0 / 1416	-28.0	-28.0	0.44 (2)	10.00
I-H	0 / 0	-28.0	-28.0	0.27 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L-C	-49 / 204	0.05 (3)
B-L	0 / 1263	0.28 (1)
I-G	0 / 1778	0.40 (1)
C-K	0 / 702	0.16 (1)
I-F	-877 / 0	0.34 (1)
K-D	-566 / 0	0.22 (1)
K-F	0 / 484	0.11 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(65 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.57/1.00 (G-H:1) , BC=0.44/1.00 (I-K:2) , WB=0.40/1.00 (G-I:1) , SSI=0.25/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (K) (INPUT = 0.90)
JSI METAL= 0.44 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEERS
1167
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 3988-13
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285994	T10	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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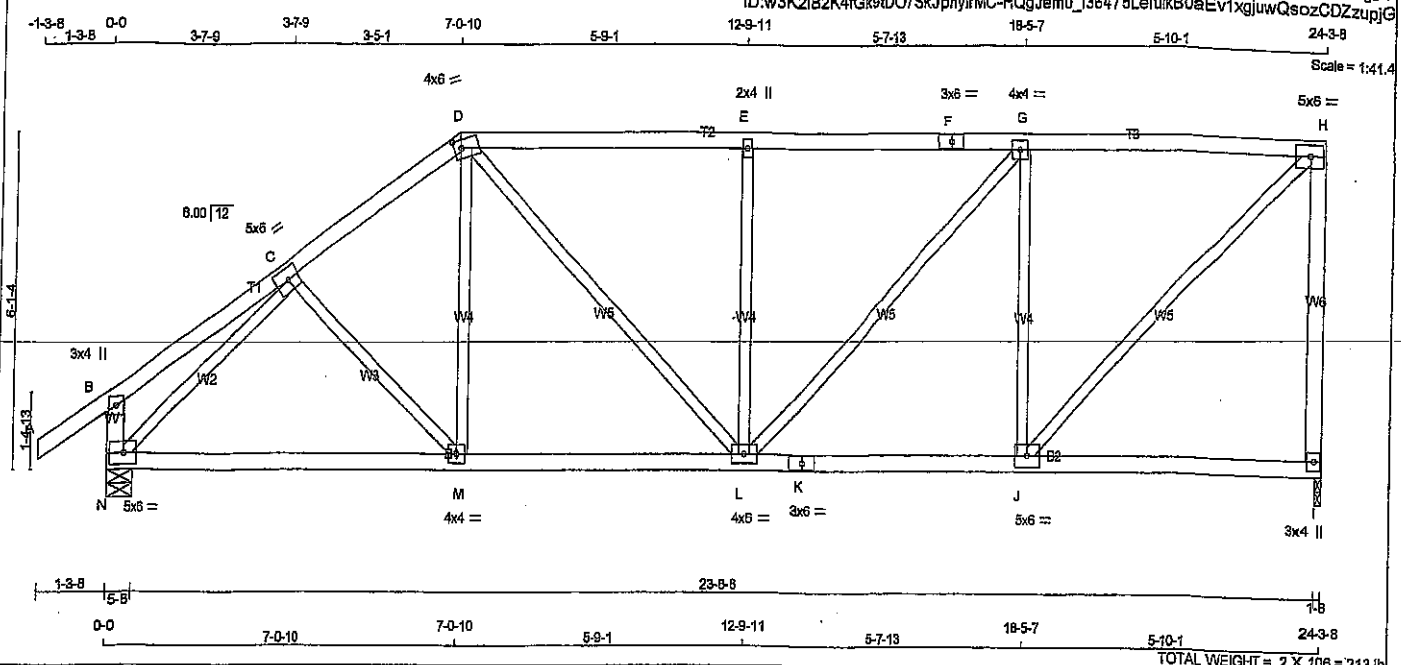

TOTAL WEIGHT = 2 X 106 = 213 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW-m	MT20	4.0	6.0	1.75	1.75
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TMW-t	MT20	5.0	6.0		
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	5.0	6.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	4.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	1363	0	1363	0	0	1-8	1-8
N	1479	0	1479	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1119	821 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
N	1198	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00
B-C	0 / 17	-84.3	-84.3	0.15 (1)	10.00
C-D	-1460 / 0	-84.3	-84.3	0.16 (1)	5.22
D-E	-1473 / 0	-84.3	-84.3	0.43 (1)	4.86
E-F	-1473 / 0	-84.3	-84.3	0.44 (1)	4.86
F-G	-1473 / 0	-84.3	-84.3	0.44 (1)	4.86
G-H	-1103 / 0	-84.3	-84.3	0.42 (1)	5.46
I-H	-1298 / 0	0.0	0.0	0.94 (1)	7.08
N-B	-233 / 0	0.0	0.0	0.02 (1)	7.81
N-M	0 / 1196	-28.0	-28.0	0.45 (2)	10.00
M-L	0 / 1202	-28.0	-28.0	0.47 (2)	10.00
L-K	0 / 1103	-28.0	-28.0	0.37 (2)	10.00
K-J	0 / 1103	-28.0	-28.0	0.37 (2)	10.00
J-I	0 / 0	-28.0	-28.0	0.24 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-M	0 / 118	0.03 (3)
M-D	0 / 307	0.07 (2)
N-C	-1689 / 0	0.84 (1)
J-H	0 / 1557	0.35 (1)
D-L	0 / 385	0.09 (1)
J-G	-912 / 0	0.53 (1)
L-E	-520 / 0	0.30 (1)
L-G	0 / 530	0.12 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.94/1.00 (H-I:1) , BC=0.47/1.00 (L-M:2) , WB=0.84/1.00 (C-N:1) , SSI=0.23/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (D) (INPUT = 0.90)
JSI METAL= 0.41 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
1678
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 3989 -18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285994	T11	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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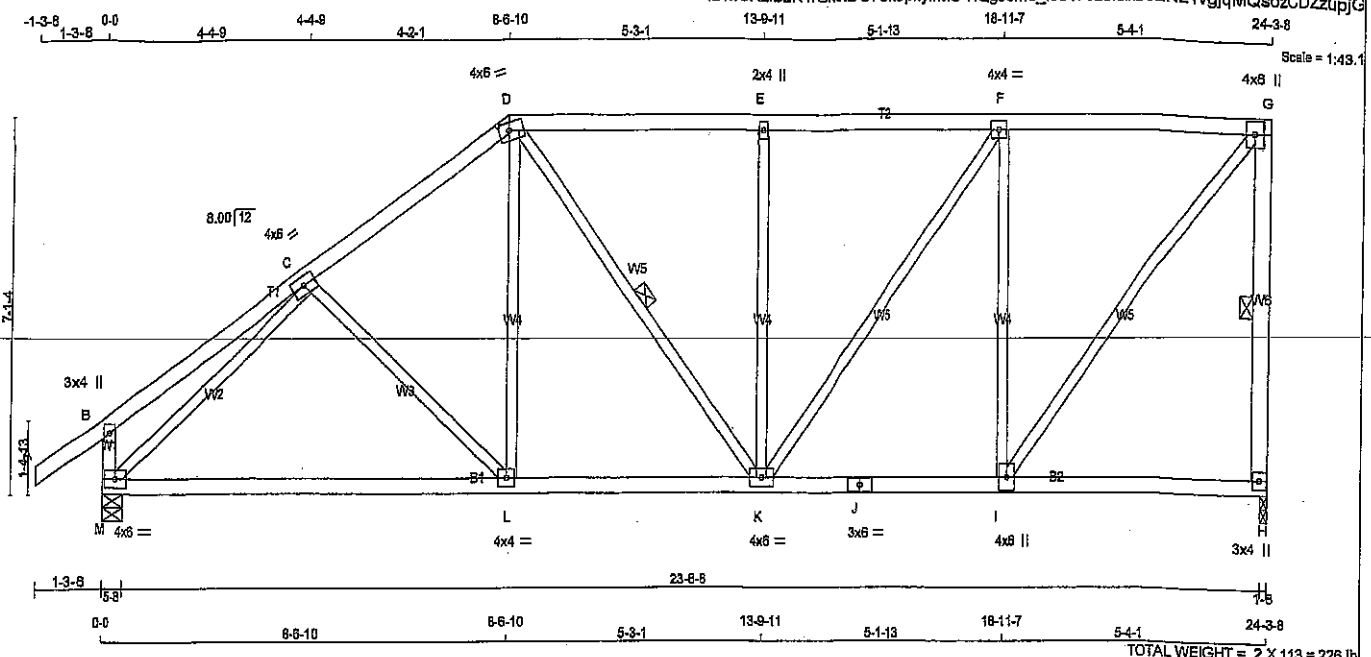

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW-m	MT20	4.0	6.0	1.75	2.50
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW+p	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWW+t	MT20	4.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQR'D BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	1363	0	1363	0	0	1-8	1-8
M	1479	0	1479	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1119	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
M	1196	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-H, D-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	C-L	-117 / 65	0.07 (1)	
B-C	0 / 23	-84.3	-84.3	0.24 (1)	10.00	L-D	0 / 414	0.09 (2)	
C-D	-1390 / 0	-84.3	-84.3	0.28 (1)	5.20	M-C	-1661 / 0	0.93 (1)	
D-E	-1226 / 0	-84.3	-84.3	0.34 (1)	5.37	I-G	0 / 1426	0.32 (1)	
E-F	-1227 / 0	-84.3	-84.3	0.35 (1)	5.35	D-K	-8 / 141	0.03 (1)	
F-G	-879 / 0	-84.3	-84.3	0.34 (1)	8.07	I-F	-946 / 0	0.63 (1)	
H-G	-1305 / 0	0.0	0.0	0.30 (1)	5.66	K-E	-474 / 0	0.41 (1)	
M-B	-255 / 0	0.0	0.0	0.03 (1)	7.81	K-F	0 / 576	0.13 (1)	
M-L	0 / 1223	-28.0	-28.0	0.59 (2)	10.00				
L-K	0 / 1141	-28.0	-28.0	0.59 (2)	10.00				
K-J	0 / 879	-28.0	-28.0	0.31 (2)	10.00				
J-I	0 / 879	-28.0	-28.0	0.31 (2)	10.00				
I-H	0 / 0	-28.0	-28.0	0.21 (3)	10.00				

LICENSED PROFESSIONAL ENGINEERS
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.23")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 779 (0.37")

CSI: TC=0.35/1.00 (E-F:1) , BC=0.59/1.00 (K-L:2) , WB=0.93/1.00 (C-M:1) , SSI=0.21/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BENC=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.90 (M) (INPUT = 0.90)
JSI METAL= 0.40 (C) (INPUT = 1.00)

DWG NO. TAM 3990 -18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43554	DRWG NO.
285994	T12	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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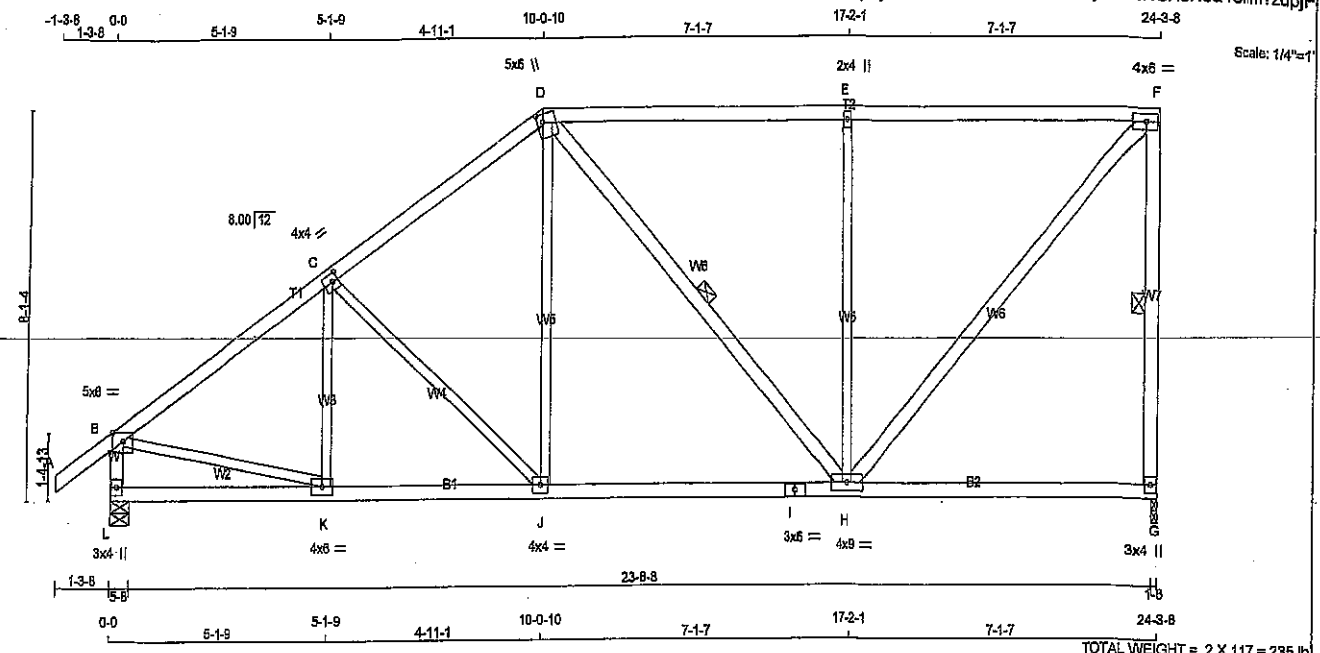

TOTAL WEIGHT = 2 X 117 = 235 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1363	0	1363	0	0	1-8	1-8
L	1479	0	1479	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1119	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
L	1198	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.61 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 32	-84.3	-84.3	0.11 (1)		10.00	K-C	-123 / 133	0.04 (1)
B-C	-1529 / 0	-84.3	-84.3	0.43 (1)		4.83	C-J	-347 / 0	0.29 (1)
C-D	-1275 / 0	-84.3	-84.3	0.40 (1)		5.20	J-D	0 / 436	0.10 (2)
D-E	-967 / 0	-84.3	-84.3	0.60 (1)		4.81	D-H	-110 / 0	0.06 (1)
E-F	-957 / 0	-84.3	-84.3	0.81 (1)		4.81	H-E	-745 / 0	0.94 (1)
G-F	-1284 / 0	0.0	0.0	0.38 (1)		5.59	H-F	0 / 1432	0.23 (1)
L-B	-1418 / 0	0.0	0.0	0.15 (1)		6.85	B-K	0 / 1323	0.30 (1)
L-K	0 / 0	-28.0	-28.0	0.16 (3)		10.00			
K-J	0 / 1295	-28.0	-28.0	0.33 (2)		10.00			
J-I	0 / 1041	-28.0	-28.0	0.49 (2)		10.00			
I-H	0 / 1041	-28.0	-28.0	0.49 (2)		10.00			
H-G	0 / 0	-28.0	-28.0	0.37 (3)		10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.81/1.00 (E-F:1) , BC=0.49/1.00 (H-J:2) , WB=0.94/1.00 (E-H:1) , SSI=0.29/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	522	2284	1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL= 0.36 (i) (INPUT = 1.00)

DWG NO. TAM 3991-12
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
286039	T13	5	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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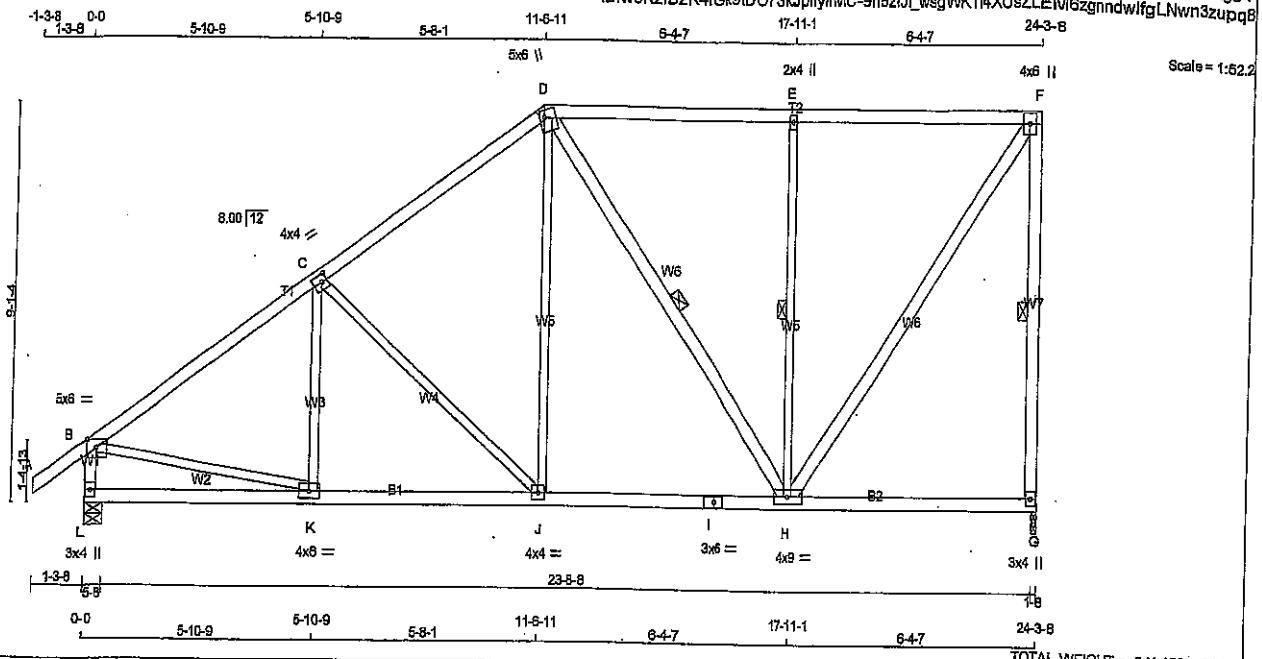

TOTAL WEIGHT = 5 X 123 = 614 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1363	0	1363	0	0	1-8	1-8
L	1479	0	1479	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1119	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
L	1198	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.61 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	
B-C	-1531 / 0	-84.3	-84.3	0.59 (1)	4.61	
C-D	-1167 / 0	-84.3	-84.3	0.54 (1)	5.16	
D-E	-790 / 0	-84.3	-84.3	0.61 (1)	5.73	
E-F	-790 / 0	-84.3	-84.3	0.62 (1)	5.73	
G-F	-1292 / 0	0.0	0.0	0.50 (1)	5.68	
L-B	-1412 / 0	0.0	0.0	0.15 (1)	6.86	
L-K	0 / 0	-28.0	-28.0	0.23 (3)	10.00	
K-J	0 / 1300	-28.0	-28.0	0.38 (2)	10.00	
J-I	0 / 946	-28.0	-28.0	0.40 (2)	10.00	
I-H	0 / 946	-28.0	-28.0	0.40 (2)	10.00	
H-G	0 / 0	-28.0	-28.0	0.29 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K-C	-58 / 203	0.05 (3)
C-J	-474 / 0	0.58 (1)
J-D	0 / 490	0.11 (2)
D-H	-270 / 0	0.17 (1)
H-E	-866 / 0	0.38 (1)
H-F	0 / 1344	0.22 (1)
B-K	0 / 1322	0.30 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.62/1.00 (E-F:1) , BC=0.40/1.00 (H-J:2) , WB=0.56/1.00 (C-J:1) , SSI=0.26/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1667	822	2284	1858

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.32 (K) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
12676
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 3993 -13
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285934	T14	6	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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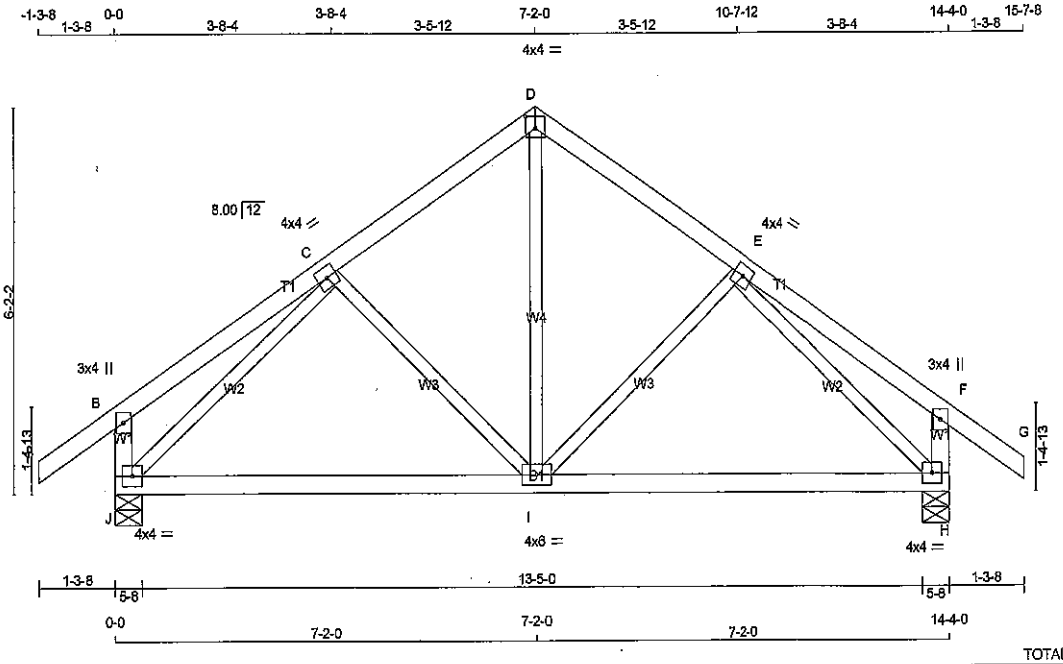

TOTAL WEIGHT = 6 X 62 = 371 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	920	0	920	0	0	5-8	5-8
H	920	0	920	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	739	437 / 0	151 / 0	0 / 0	0 / 0	152 / 0	0 / 0
H	739	437 / 0	151 / 0	0 / 0	0 / 0	152 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.3	-84.3	1	0.11 (1)	10.00
B- C	0 / 19	-84.3	-84.3	1	0.17 (1)	10.00
C- D	-545 / 0	-84.3	-84.3	1	0.13 (1)	6.25
D- E	-545 / 0	-84.3	-84.3	1	0.13 (1)	6.25
E- F	0 / 19	-84.3	-84.3	1	0.17 (1)	10.00
F- G	0 / 32	-84.3	-84.3	1	0.11 (1)	10.00
J- B	-232 / 0	0.0	0.0	1	0.02 (1)	7.81
H- F	-232 / 0	0.0	0.0	1	0.02 (1)	7.81
J- I	0 / 631	-28.0	-28.0	2	0.49 (2)	10.00
I- H	0 / 631	-28.0	-28.0	2	0.49 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- D	0 / 460	0.10 (1)
I- E	-152 / 44	0.06 (1)
C- I	-152 / 44	0.06 (1)
J- C	-879 / 0	0.35 (1)
E- H	-879 / 0	0.35 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.48")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.48")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.17/1.00 (B-C:1) , BC=0.49/1.00 (I-J:2) , WB=0.35/1.00 (E-H:1) , SSI=0.17/1.00 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (D) (INPUT = 0.90)
JSI METAL= 0.31 (E) (INPUT = 1.00)

DWG NO. TAM 21155 -18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285929	T16X	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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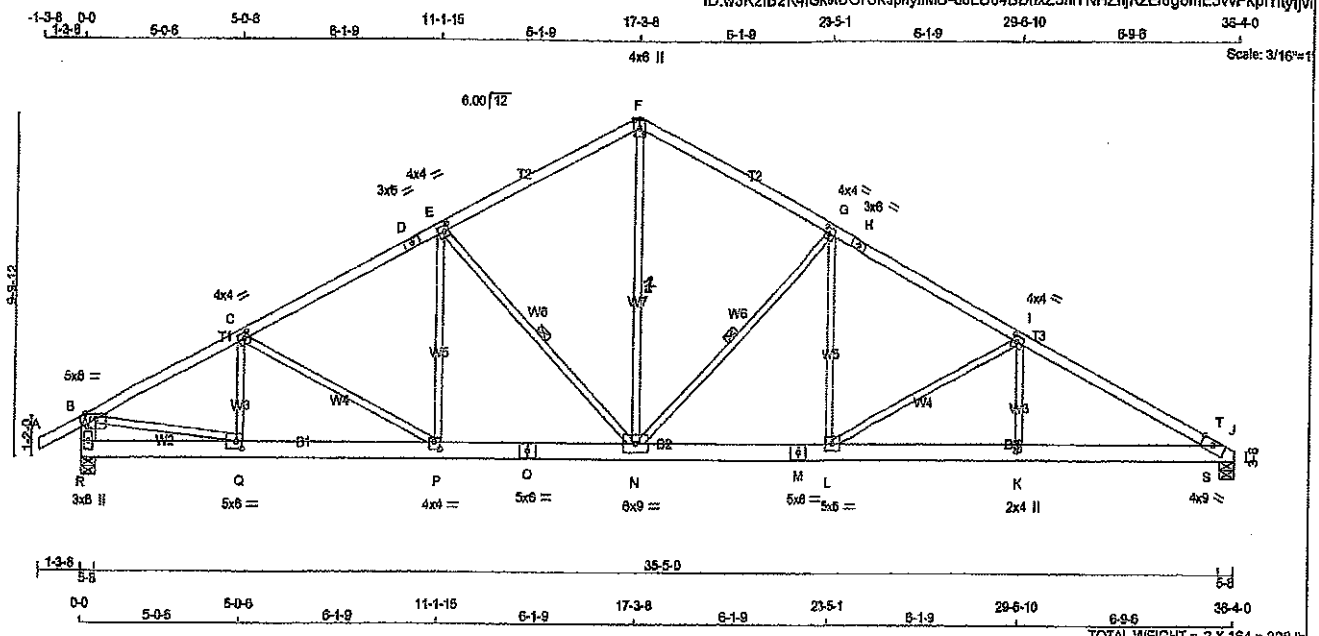


LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESGR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
R - Q	2x6	DRY	No.2	SPF
O - M	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C, E, G, I						
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	6.0		
F	TTW+p	MT20	4.0	6.0	Edge	
H	TS-t	MT20	3.0	6.0		
J	TBM1-h	MT20	4.0	9.0	Edge	
K	BMW+w	MT20	2.0	4.0	2.50	1.00
L	BMWW-t	MT20	5.0	6.0		
M	BS-t	MT20	5.0	6.0		
N	BMWWW-t	MT20	6.0	9.0		
O	BS-t	MT20	5.0	6.0		
P	BMWW-t	MT20	4.0	4.0	2.50	2.00
Q	BMWW-t	MT20	5.0	6.0	2.50	2.00
R	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
R	2154	0	2154	0	0	5-8	5-8
J	2039	0	2039	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LOASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1752	899 / 0	362 / 0	0 / 0	0 / 0	371 / 0	0 / 0
J	1674	930 / 0	362 / 0	0 / 0	0 / 0	383 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.12 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

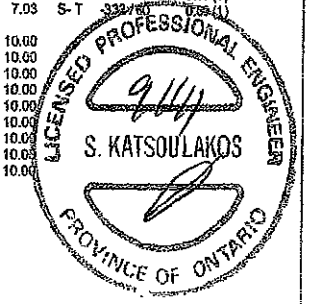
1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N, G-N. FNI

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-84.3	-84.3	0.11 (1)		10.00
B- C	-3052 / 0	-84.3	-84.3	0.40 (1)		3.70
C- D	-2834 / 0	-84.3	-84.3	0.53 (1)		3.70
D- E	-2834 / 0	-84.3	-84.3	0.53 (1)		3.70
E- F	-2230 / 0	-84.3	-84.3	0.48 (1)		4.13
F- G	-2229 / 0	-84.3	-84.3	0.48 (1)		4.13
G- H	-3040 / 0	-84.3	-84.3	0.55 (1)		3.59
H- I	-3040 / 0	-84.3	-84.3	0.55 (1)		3.59
I- T	-3048 / 0	-84.3	-84.3	0.65 (1)		3.12
T- J	-3927 / 0	-84.3	-84.3	0.24 (1)		3.41
R- B	-2079 / 0	0.0	0.0	0.13 (1)		7.03
R- Q	0 / 0	-28.0	-28.0	0.06 (2)		10.00
Q- P	0 / 2747	-28.0	-28.0	0.40 (1)		10.00
P- O	0 / 2542	-28.0	-28.0	0.37 (1)		10.00
O- N	0 / 2542	-28.0	-28.0	0.37 (1)		10.00
N- M	0 / 2723	-28.0	-28.0	0.39 (1)		10.00
M- L	0 / 2723	-28.0	-28.0	0.39 (1)		10.00
L- K	0 / 3544	-28.0	-28.0	0.52 (1)		10.00
K- S	0 / 3544	-28.0	-28.0	0.62 (1)		10.00
S- J	0 / 3482	-28.0	-28.0	0.81 (1)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-283 / 73	0.06 (1)
C- P	-224 / 0	0.21 (1)
P- E	0 / 387	0.09 (2)
E- N	-819 / 0	0.43 (1)
N- F	0 / 1561	0.35 (1)
N- G	-1080 / 0	0.57 (1)
L- G	0 / 596	0.13 (2)
L- I	-933 / 0	0.65 (1)
K- I	0 / 408	0.09 (2)
B- Q	0 / 2780	0.63 (1)
S- T	-332 / 60	0.09 (1)



DWG NO. TAM 45495-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.21")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.21")
ALLOWABLE DEFL.(TL)= L/360 (1.21")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.35")

CSI: TC=0.65 (I-T:1) , BC=0.82 (K-S:1) , WB=0.65 (I-L:1) , SSI=0.45 (J-S:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.60

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1556

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (Q) (INPUT = 0.90)
JSI METAL= 0.77 (J) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285929	T17S	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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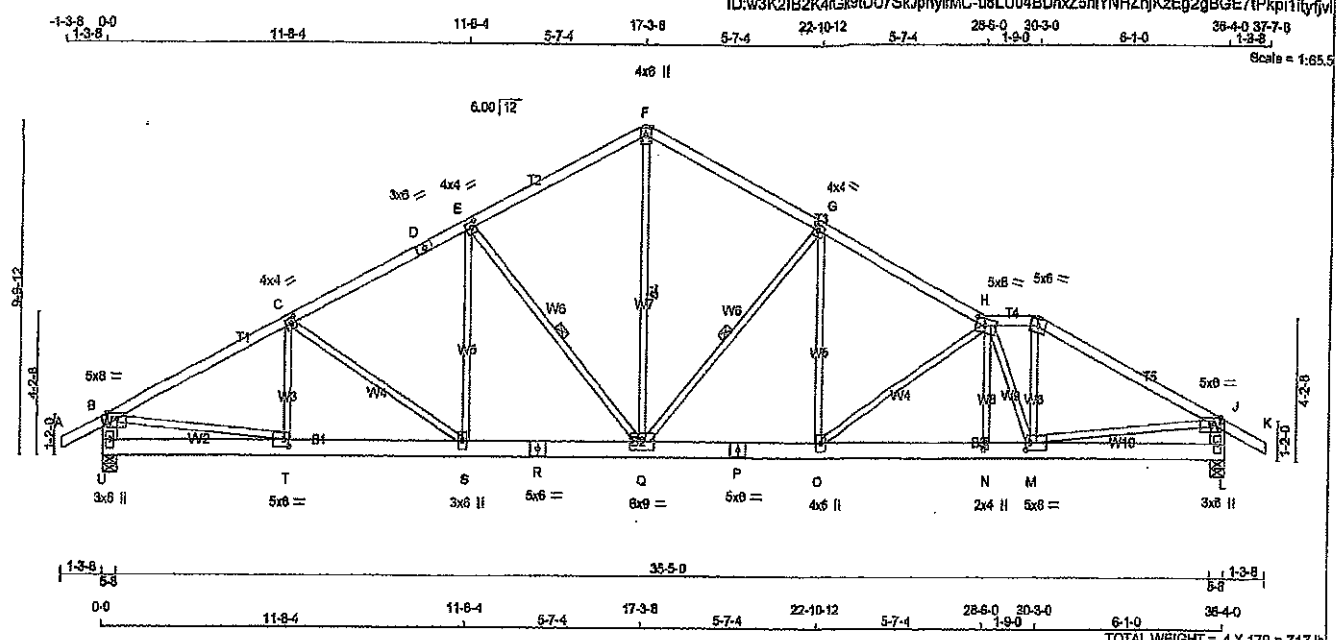

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - I	2x4	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
U - R	2x6	DRY	No.2	SPF
R - P	2x6	DRY	No.2	SPF
P - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C, E, G						
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	6.0		
F	TTW+p	MT20	4.0	8.0	Edge	
H	TTWWW-m	MT20	5.0	8.0	2.50	3.50
I	TTW-m	MT20	5.0	8.0		
J	TMVW-p	MT20	5.0	8.0	Edge	
L	BMV1+p	MT20	3.0	6.0		
M	BMWWW-t	MT20	5.0	8.0	2.50	1.50
N	BMW+w	MT20	2.0	4.0	2.50	1.00
O	BMWW+t	MT20	4.0	6.0		
P	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	6.0	8.0		
R	BS-t	MT20	5.0	6.0		
S	BMWW+t	MT20	3.0	6.0		
T	BMWW-t	MT20	6.0	8.0	2.50	2.00
U	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	2154	0	2154	0	0	5-8	5-8
L	2154	0	2154	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1752	999 / 0	382 / 0	0 / 0	0 / 0	371 / 0	0 / 0
L	1752	999 / 0	382 / 0	0 / 0	0 / 0	371 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.49 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-Q, G-Q. Fig 2

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	CSI	LC	MAX. UNBRAC LENGTH
A- B	0 / 26	-84.3	-84.3	0.11	(1)	10.00
B- C	-3085 / 0	-84.3	-84.3	0.56	(1)	3.54
C- D	-2762 / 0	-84.3	-84.3	0.44	(1)	3.62
D- E	-2762 / 0	-84.3	-84.3	0.44	(1)	3.62
E- F	-2224 / 0	-84.3	-84.3	0.40	(1)	4.22
F- G	-2229 / 0	-84.3	-84.3	0.42	(1)	4.17
G- H	-2952 / 0	-84.3	-84.3	0.49	(1)	3.64
H- I	-2761 / 0	-84.3	-84.3	0.11	(1)	4.11
I- J	-3040 / 0	-84.3	-84.3	0.60	(1)	3.49
J- K	0 / 26	-84.3	-84.3	0.11	(1)	10.00
U- B	-2059 / 0	0.0	0.0	0.13	(1)	7.05
L- J	-2070 / 0	0.0	0.0	0.13	(1)	7.05
U- T	0 / 0	-28.0	-28.0	0.11	(2)	10.00
T- S	0 / 2777	-28.0	-28.0	0.39	(1)	10.00
S- R	0 / 2487	-28.0	-28.0	0.35	(1)	10.00
R- Q	0 / 2487	-28.0	-28.0	0.35	(1)	10.00
Q- P	0 / 2664	-28.0	-28.0	0.37	(1)	10.00
P- O	0 / 2664	-28.0	-28.0	0.37	(1)	10.00
O- N	0 / 3258	-28.0	-28.0	0.47	(1)	10.00
N- M	0 / 3259	-28.0	-28.0	0.43	(1)	10.00
M- L	0 / 0	-28.0	-28.0	0.12	(2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI	LC
T- C	-204 / 130	0.05	(1)
S- E	0 / 412	0.09	(2)
E- Q	-799 / 0	0.40	(1)
Q- F	0 / 1600	0.38	(1)
Q- G	-1073 / 0	0.54	(1)
O- G	0 / 600	0.14	(2)
O- H	-718 / 0	0.62	(1)
N- H	0 / 82	0.02	(3)
H- M	-1297 / 0	0.38	(1)
M- I	0 / 1033	0.23	(1)
B- T	0 / 2881	0.33	(1)
M- J	0 / 2764	0.31	(1)
C- S	-346 / 0	0.30	(1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.21")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.20")
ALLOWABLE DEFL.(TL)= L/360 (1.21")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.33")

CSI: TC=0.60 (I-J:1) , BC=0.47 (N-O:1) , WB=0.63 (B-T:1) , SSI=0.21 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1655

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (M) (INPUT = 0.90)
JSI METAL= 0.70 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. PAM45249617
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285929	T18A	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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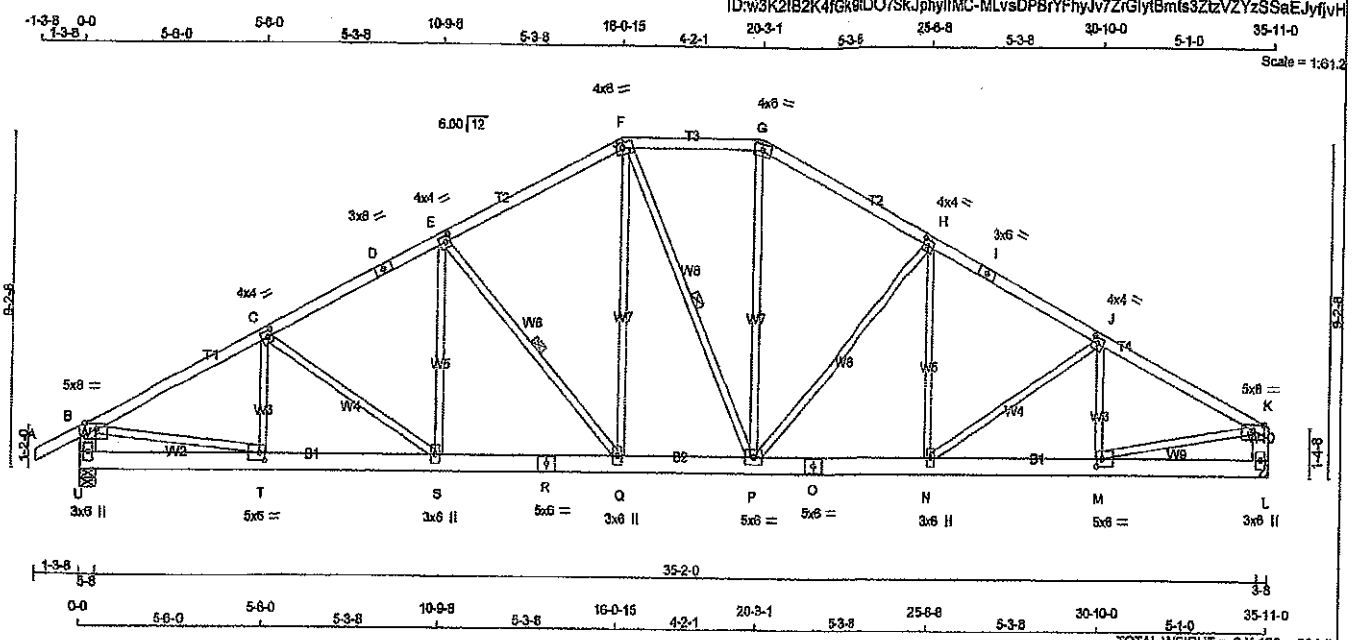

TOTAL WEIGHT = 2 X 180 = 361 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
L - K	2x6	DRY	No.2	SPF
U - R	2x6	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW-p	MT20	5.0	8.0	Edge	
C, E, H, J						
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	6.0		
F	TTWW-m	MT20	4.0	6.0	1.75	2.25
G	TTW-m	MT20	4.0	6.0		
I	TS-t	MT20	3.0	6.0		
K	TMW-p	MT20	5.0	8.0	Edge	
L	BMV1+p	MT20	3.0	6.0		
M	BMWW-t	MT20	5.0	6.0	2.50	2.00
N, O, S						
N	BMWW+t	MT20	3.0	6.0		
O	BS-t	MT20	5.0	6.0		
P	BMWWW-t	MT20	5.0	6.0		
R	BS-t	MT20	5.0	6.0		
T	BMWW-t	MT20	5.0	6.0	2.50	2.00
U	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
U	2130	0	2130	0	0	IN-SX	IN-SX
L	2016	0	2016	0	0	5-8	5-8
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1793	988 / 0	377 / 0	0 / 0	0 / 0	397 / 0	0 / 0
L	1655	919 / 0	377 / 0	0 / 0	0 / 0	359 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.65 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-Q, F-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF)	CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 26	-84.3	-84.3	0.11 (1)	10.00	
B- C	-3023 / 0	-84.3	-84.3	0.45 (1)	3.65	
C- D	-2807 / 0	-84.3	-84.3	0.39 (1)	3.85	
D- E	-2807 / 0	-84.3	-84.3	0.38 (1)	3.86	
E- F	-2284 / 0	-84.3	-84.3	0.33 (1)	4.22	
F- G	-2027 / 0	-84.3	-84.3	0.24 (1)	4.54	
G- H	-2279 / 0	-84.3	-84.3	0.36 (1)	4.22	
H- I	-2740 / 0	-84.3	-84.3	0.38 (1)	3.90	
I- J	-2740 / 0	-84.3	-84.3	0.38 (1)	3.90	
J- K	-2831 / 0	-84.3	-84.3	0.36 (1)	3.84	
U- B	-2049 / 0	0.0	0.0	0.13 (1)	7.07	
L- K	-1941 / 0	0.0	0.0	0.13 (1)	7.22	
U- T	0 / 0	-28.0	-28.0	0.10 (2)	10.00	
T- S	0 / 2720	-28.0	-28.0	0.36 (1)	10.00	
S- R	0 / 2511	-28.0	-28.0	0.34 (1)	10.00	
R- Q	0 / 2511	-28.0	-28.0	0.34 (1)	10.00	
Q- P	0 / 2033	-28.0	-28.0	0.29 (1)	10.00	
P- O	0 / 2454	-28.0	-28.0	0.34 (1)	10.00	
O- N	0 / 2454	-28.0	-28.0	0.34 (1)	10.00	
N- M	0 / 2548	-28.0	-28.0	0.36 (1)	10.00	
M- L	0 / 0	-28.0	-28.0	0.09 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- C	-250 / 86	0.06 (1)
C- S	-252 / 0	0.18 (1)
S- E	0 / 377	0.08 (2)
E- Q	-738 / 0	0.33 (1)
Q- F	0 / 672	0.15 (1)
F- P	-13 / 0	0.01 (1)
P- G	0 / 658	0.15 (1)
P- H	-660 / 0	0.92 (1)
N- H	0 / 311	0.07 (2)
N- J	-113 / 0	0.08 (1)
M- J	-360 / 20	0.08 (1)
B- T	0 / 2747	0.62 (1)
M- K	0 / 2695	0.09 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.8	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.26")

CSI: TC=0.46 (B-C:1) , BC=0.38 (S-T:1) , WB=0.92 (H-P:1) , SSI=0.19 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (M) (INPUT = 0.90)
JSI METAL= 0.68 (T) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
9/11/17
S. KATSOULAROS
PROVINCE OF ONTARIO

DWG NO. TAM4549717
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285929	T19A	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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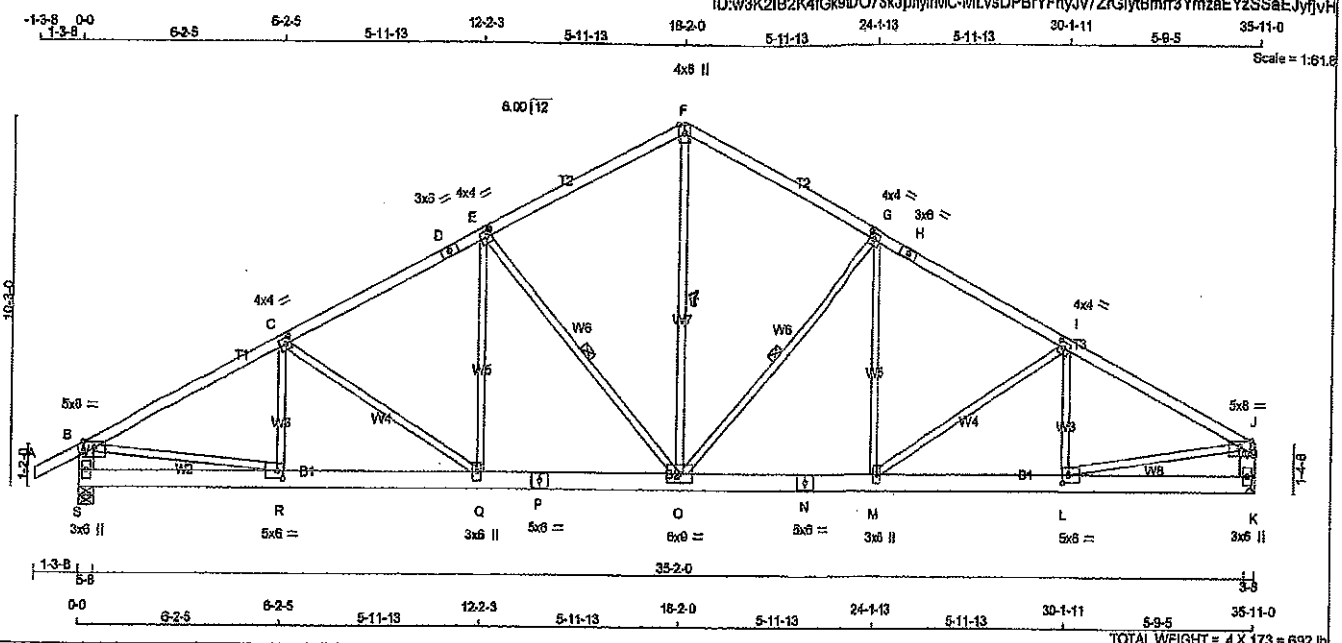


TOTAL WEIGHT = 4 X 173 = 692 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
K - J	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	5.0	6.0	Edge	
C, E, G, I						
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	6.0		
F	TTW+p	MT20	4.0	6.0	Edge	
H	TS-t	MT20	3.0	6.0		
J	TMWW-p	MT20	5.0	6.0	Edge	
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.00
M	BMWW+t	MT20	3.0	6.0		
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	6.0	9.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWW+t	MT20	3.0	6.0		
R	BMWW-t	MT20	5.0	6.0	2.50	2.00
S	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT	REQRD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	BRG
S	2130	0	2130	0	0	IN-SX	IN-SX
K	2016	0	2016	0	0	5-8	5-8
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1733	988 / 0	377 / 0	0 / 0	0 / 0	367 / 0	0 / 0
K	1655	919 / 0	377 / 0	0 / 0	0 / 0	359 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.52 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

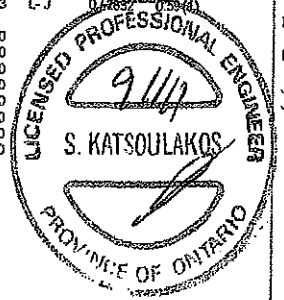
1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-O, G-O. F-O

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 26	-84.3	-84.3	0.11 (1)		10.00
B-C	-3050 / 0	-84.3	-84.3	0.59 (1)		3.52
C-D	-2899 / 0	-84.3	-84.3	0.47 (1)		3.85
D-E	-2899 / 0	-84.3	-84.3	0.47 (1)		3.85
E-F	-2085 / 0	-84.3	-84.3	0.44 (1)		4.28
F-G	-2086 / 0	-84.3	-84.3	0.44 (1)		4.28
G-H	-2852 / 0	-84.3	-84.3	0.47 (1)		3.86
H-I	-2852 / 0	-84.3	-84.3	0.47 (1)		3.86
I-J	-2861 / 0	-84.3	-84.3	0.49 (1)		3.70
S-B	-2045 / 0	0.0	0.0	0.13 (1)		7.08
K-J	-1937 / 0	0.0	0.0	0.13 (1)		7.23
S-R	0 / 0	-28.0	-28.0	0.12 (2)		10.00
R-Q	0 / 2747	-28.0	-28.0	0.39 (1)		10.00
Q-P	0 / 2414	-28.0	-28.0	0.35 (1)		10.00
P-O	0 / 2414	-28.0	-28.0	0.35 (1)		10.00
O-N	0 / 2374	-28.0	-28.0	0.34 (1)		10.00
N-M	0 / 2374	-28.0	-28.0	0.34 (1)		10.00
M-L	0 / 2595	-28.0	-28.0	0.37 (1)		10.00
L-K	0 / 0	-28.0	-28.0	0.10 (2)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R-C	-179 / 152	0.05 (1)
C-Q	-397 / 0	0.38 (1)
Q-E	0 / 446	0.10 (2)
E-O	-864 / 0	0.49 (1)
O-F	0 / 1439	0.32 (1)
O-G	-804 / 0	0.46 (1)
M-G	0 / 395	0.09 (2)
M-I	-264 / 0	0.25 (1)
L-I	-262 / 89	0.07 (1)
B-R	0 / 2768	0.62 (1)
L-J	0 / 2837	0.59 (1)



DWG NO. TAM-454961?
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.27")

CSI: TC=0.59 (B-C:1) , BC=0.39 (Q-R:1) , WB=0.62 (B-R:1) , SSI=0.22 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	622	2284	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (O) (INPUT = 0.90)
JSI METAL= 0.69 (R) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285929	T20A	6	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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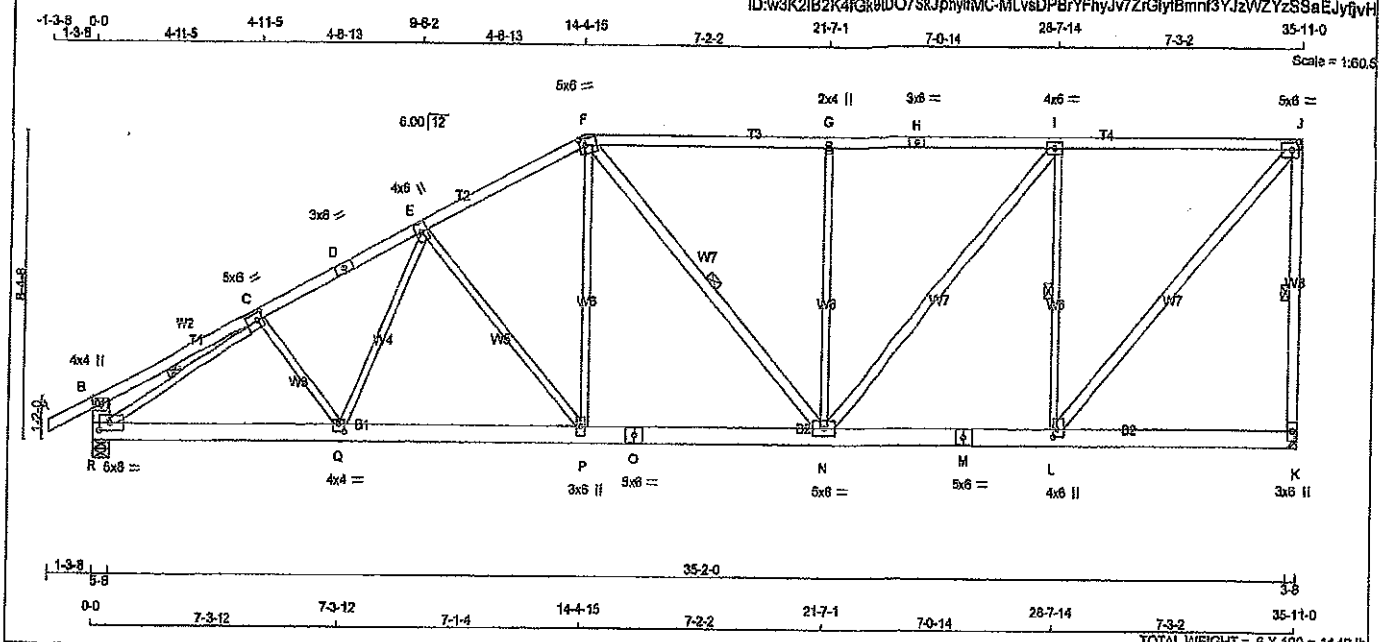

TOTAL WEIGHT = 6 X 190 = 1142 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
O - M	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
F - N	2x4	DRY	No.2	SPF
N - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.00
D	TS-t	MT20	3.0	6.0		
E	TMWW+t	MT20	4.0	6.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMV+w	MT20	2.0	4.0		
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	4.0	6.0		
J	TMVV-t	MT20	5.0	6.0	2.50	2.25
K	BMV1+p	MT20	3.0	6.0		
L	BMWW+t	MT20	4.0	6.0	2.50	1.50
M	BS-t	MT20	5.0	6.0		
N	BMWWW-t	MT20	5.0	6.0		
O	BS-t	MT20	5.0	6.0		
P	BMWW+t	MT20	3.0	6.0		
Q	BMWW-t	MT20	4.0	4.0	2.50	2.00
R	BMVW1-t	MT20	5.0	6.0	2.50	3.25

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
K	2018	0	2018	0	0	HANGER BY OTHERS IN-SX	IN-SX
						MIN. SEAT SIZE: 3-8	
R	2130	0	2130	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	1855	919 / 0	377 / 0	0 / 0	0 / 0	359 / 0	0 / 0
R	1733	988 / 0	377 / 0	0 / 0	0 / 0	367 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.60 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF J-K, C-R, F-N, I-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 26	-84.3	-84.3	0.11 (1)	10.00
B-C	0 / 15	-84.3	-84.3	0.24 (1)	10.00
C-D	-2959 / 0	-84.3	-84.3	0.44 (1)	3.70
D-E	-2959 / 0	-84.3	-84.3	0.44 (1)	3.70
E-F	-2463 / 0	-84.3	-84.3	0.38 (1)	4.05
F-G	-2231 / 0	-84.3	-84.3	0.85 (1)	3.85
G-H	-2231 / 0	-84.3	-84.3	0.85 (1)	3.80
H-I	-2231 / 0	-84.3	-84.3	0.85 (1)	3.80
I-J	-1531 / 0	-84.3	-84.3	0.75 (1)	4.32
K-J	-1933 / 0	0.0	0.0	0.59 (1)	4.84
R-B	-284 / 0	0.0	0.0	0.02 (1)	7.81
R-O	0 / 2655	-28.0	-28.0	0.42 (1)	10.00
Q-P	0 / 2559	-28.0	-28.0	0.38 (1)	10.00
P-O	0 / 2194	-28.0	-28.0	0.34 (1)	10.00
O-N	0 / 2194	-28.0	-28.0	0.34 (1)	10.00
N-M	0 / 1531	-28.0	-28.0	0.30 (2)	10.00
M-L	0 / 1531	-28.0	-28.0	0.30 (2)	10.00
L-K	0 / 0	-28.0	-28.0	0.17 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-O	-12 / 156	0.04 (3)
Q-E	0 / 311	0.07 (2)
E-P	-571 / 0	0.61 (1)
P-F	0 / 646	0.15 (1)
R-C	-3181 / 0	0.74 (1)
L-J	0 / 2278	0.37 (1)
F-N	-6 / 56	0.01 (1)
L-I	-1457 / 0	0.62 (1)
N-G	-653 / 0	0.66 (1)
N-I	0 / 1059	0.17 (1)

DWG NO. T2045499 17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.25")

CSI: TC=0.66 (G-I:1) , BC=0.42 (O-R:1) , WB=0.66 (G-N:1) , SSI=0.26 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1697	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (R) (INPUT = 0.90)
JSI METAL= 0.79 (C) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285929	T21A	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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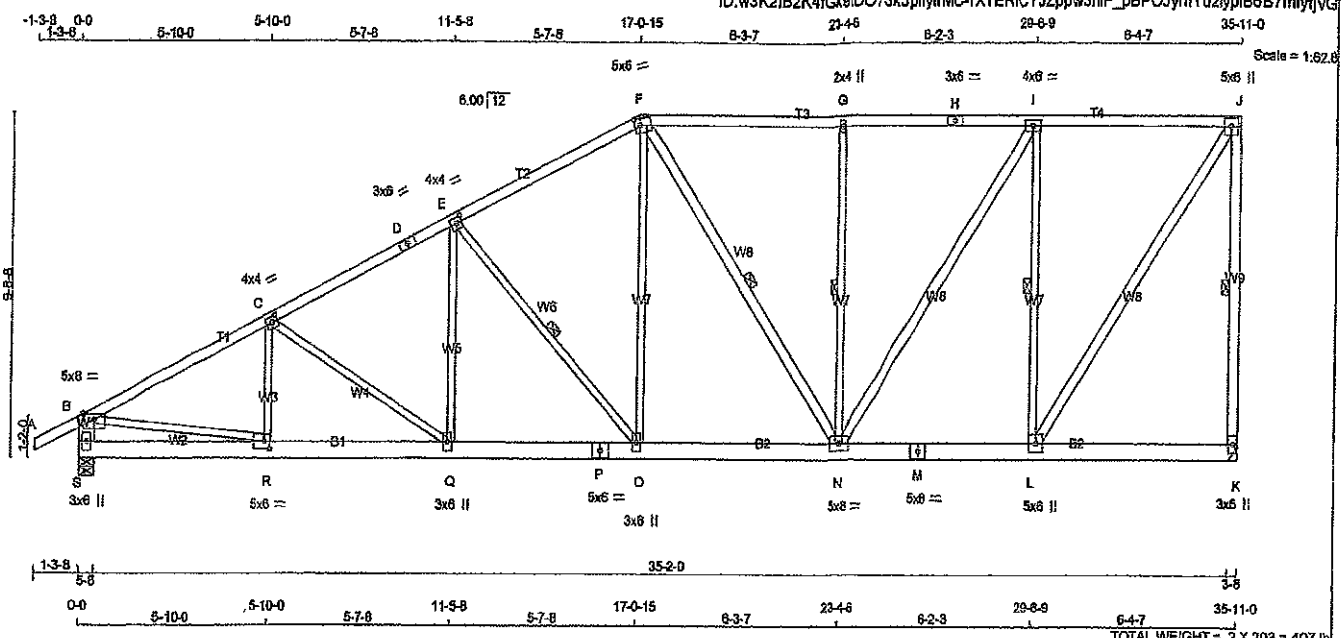

TOTAL WEIGHT = 2 X 203 = 407 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
F - N	2x4	DRY	No.2	SPF
N - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.75
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMW+w	MT20	2.0	4.0		
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	4.0	6.0		
J	TMVW+p	MT20	5.0	6.0		
K	BMV1+p	MT20	3.0	6.0		
L	BMWW+t	MT20	5.0	6.0		
M	BS-t	MT20	5.0	6.0		
N	BMWWW-t	MT20	5.0	6.0		
O	BMWW+t	MT20	3.0	6.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWW+t	MT20	3.0	6.0		
R	BMWW-t	MT20	5.0	6.0	2.50	2.00
S	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
K	2016	0	2016	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	
S	2130	0	2130	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	1655	919 / 0	377 / 0	0 / 0	0 / 0	359 / 0	0 / 0
S	1733	988 / 0	377 / 0	0 / 0	0 / 0	397 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.50 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF J-K, E-O, F-N, I-L, G-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 26	-84.3	-84.3	0.11 (1)	10.00	R- C	-214 / 119	0.05 (1)
B- C	-3039 / 0	-84.3	-84.3	0.65 (1)	3.50	C- Q	-928 / 0	0.27 (1)
C- D	-2756 / 0	-84.3	-84.3	0.59 (1)	3.89	Q- E	0 / 404	0.09 (2)
D- E	-2756 / 0	-84.3	-84.3	0.59 (1)	3.89	E- O	-797 / 0	0.40 (1)
E- F	-2168 / 0	-84.3	-84.3	0.51 (1)	4.12	O- F	0 / 777	0.17 (1)
F- G	-1803 / 0	-84.3	-84.3	0.56 (1)	4.35	B- R	0 / 2759	0.62 (1)
G- H	-1803 / 0	-84.3	-84.3	0.57 (1)	4.32	L- J	0 / 2091	0.34 (1)
H- I	-1803 / 0	-84.3	-84.3	0.57 (1)	4.32	F- N	-257 / 0	0.17 (1)
I- J	-1179 / 0	-84.3	-84.3	0.52 (1)	5.16	L- I	-1525 / 0	0.82 (1)
K- J	-1943 / 0	0.0	0.0	0.84 (1)	4.83	N- G	-574 / 0	0.35 (1)
S- B	-2047 / 0	0.0	0.0	0.13 (1)	7.08	N- I	-2131 / 0	0.18 (1)
S- R	0 / 0	-28.0	-28.0	0.11 (2)	10.00			
R- Q	0 / 2735	-28.0	-28.0	0.39 (1)	10.00			
Q- P	0 / 2465	-28.0	-28.0	0.34 (1)	10.00			
P- O	0 / 2465	-28.0	-28.0	0.34 (1)	10.00			
O- N	0 / 1947	-28.0	-28.0	0.28 (1)	10.00			
N- M	0 / 1179	-28.0	-28.0	0.23 (2)	10.00			
M- L	0 / 1179	-28.0	-28.0	0.23 (2)	10.00			
L- K	0 / 0	-28.0	-28.0	0.13 (3)	10.00			

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TRM 45500-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , SCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (1.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.25")

CSI: TC=0.84 (J-K:1) , BC=0.39 (Q-R:1) , WB=0.92 (I-L:1) , SSI=0.25 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (J) (INPUT = 0.90)
JSI METAL= 0.68 (R) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285298	T22	1	1	TRUSS DESC.	

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Tamarack Roof Truss, Burlington

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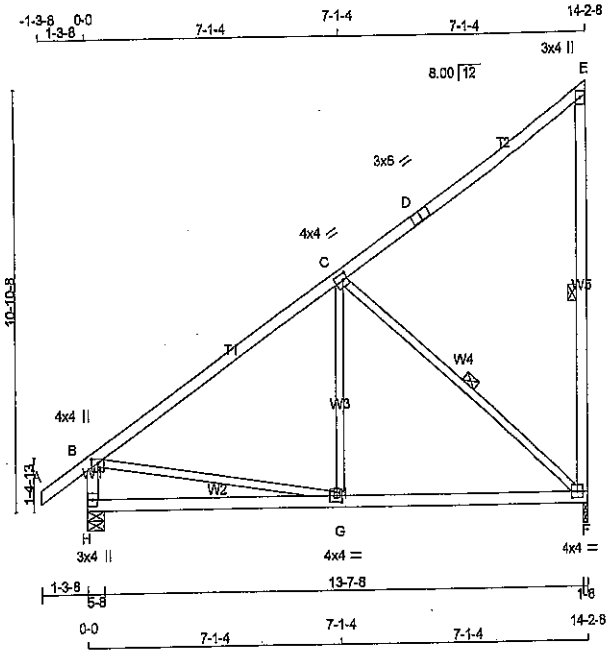

TOTAL WEIGHT = 68 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TMV+p	MT20	3.0	4.0		
F	BMVW1-t	MT20	4.0	4.0		
G	BMWW-t	MT20	4.0	4.0	2.00	1.75
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	797	0	797	0	0	1-8	1-8
H	913	0	913	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	655	364 / 0	149 / 0	0 / 0	0 / 0	142 / 0	0 / 0
H	733	434 / 0	149 / 0	0 / 0	0 / 0	150 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.16 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-F, C-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00
B-C	-688 / 0	-84.3	-84.3	0.58 (1)	6.16
C-D	-41 / 0	-84.3	-84.3	0.57 (1)	6.25
D-E	-41 / 0	-84.3	-84.3	0.57 (1)	6.25
F-E	-225 / 0	0.0	0.0	0.13 (1)	8.25
H-B	-637 / 0	0.0	0.0	0.09 (1)	7.81
H-G	0 / 0	-28.0	-28.0	0.41 (3)	10.00
G-F	0 / 606	-28.0	-28.0	0.48 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G-C	0 / 367	0.09 (3)
C-F	-764 / 0	0.44 (1)
B-G	0 / 613	0.14 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.47")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.47")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.58/1.00 (B-C:1) , BC=0.48/1.00 (F-G:2) , WB=0.44/1.00 (C-F:1) , SSI=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (G) (INPUT = 0.90)
JSI METAL= 0.23 (G) (INPUT = 1.00)

DWG NO. TAM 3350-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285929	G22	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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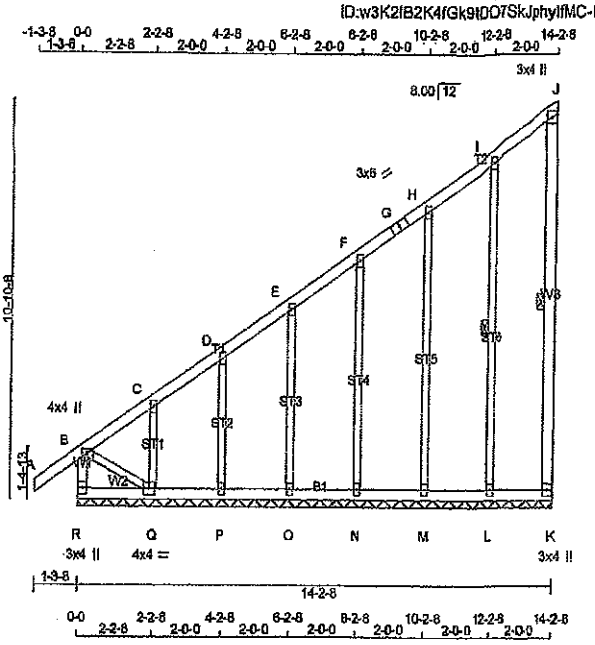

TOTAL WEIGHT = 2 X 80 = 159 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
R - K	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW+p	MT20	4.0	4.0	1.25	2.00
C, D, E, F, H, I						
C	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
J	TMV+p	MT20	3.0	4.0		
K	BMV1+p	MT20	3.0	4.0		
L, M, N, O, P						
L	BMW1+w	MT20	2.0	4.0		
Q	BMWW1-t	MT20	4.0	4.0		
R	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF J-K, H-L

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	
B-C	-11 / 0	-84.3	-84.3	0.05 (1)	6.25	
C-D	-17 / 0	-84.3	-84.3	0.05 (1)	6.25	
D-E	-9 / 0	-84.3	-84.3	0.04 (1)	10.00	
E-F	-7 / 0	-84.3	-84.3	0.04 (1)	10.00	
F-G	-4 / 0	-84.3	-84.3	0.04 (1)	10.00	
G-H	-4 / 0	-84.3	-84.3	0.04 (1)	10.00	
H-I	0 / 0	-84.3	-84.3	0.04 (1)	10.00	
I-J	-9 / 0	-84.3	-84.3	0.04 (1)	10.00	
K-J	-72 / 0	0.0	0.0	0.04 (1)	6.25	
R-B	-208 / 0	0.0	0.0	0.02 (1)	7.81	
R-O	0 / 0	-28.0	-28.0	0.03 (2)	10.00	
Q-P	0 / 11	-28.0	-28.0	0.03 (2)	10.00	
P-O	0 / 8	-28.0	-28.0	0.02 (2)	10.00	
O-N	0 / 5	-28.0	-28.0	0.02 (2)	10.00	
N-M	0 / 3	-28.0	-28.0	0.02 (2)	10.00	
M-L	0 / 2	-28.0	-28.0	0.03 (2)	10.00	
L-K	0 / 0	-28.0	-28.0	0.03 (2)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L-I	-189 / 0	0.11 (1)
M-H	-165 / 0	0.22 (1)
N-F	-167 / 0	0.13 (1)
O-E	-168 / 0	0.08 (1)
P-D	-159 / 0	0.04 (1)
Q-C	-194 / 0	0.09 (1)
B-O	0 / 20	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11 (A-B:1) , BC=0.03 (P-Q:2) , WB=0.22 (H-M:1) , SSI=0.08 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1637	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (B) (INPUT = 0.90)
JSI METAL= 0.05 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 4553617
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285298	T23	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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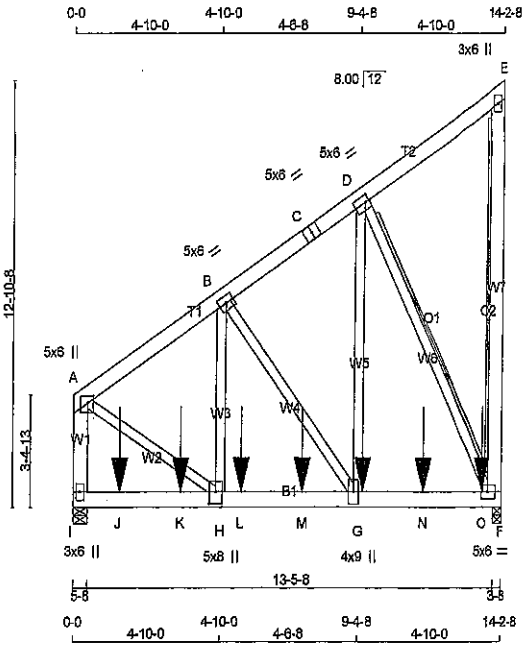

Scale = 1:70.1

TOTAL WEIGHT = 3 X 124 = 372 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
F - E	2x6	DRY	No.2	SPF
I - A	2x6	DRY	No.2	SPF
I - F	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	TOP
C- E	2	12	TOP
E- F	2	12	SIDE(33.7)
I- A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
I- F	2	7	SIDE(597.2)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW+p	MT20	5.0	8.0	2.50	2.25
B	TMWW-t	MT20	5.0	6.0		
C	TS-t	MT20	5.0	6.0		
D	TMWW-t	MT20	5.0	8.0	2.50	1.50
E	TMV+p	MT20	3.0	6.0		
F	BMVW1-t	MT20	5.0	6.0		
G	BMWW+t	MT20	4.0	9.0		
H	BMWW+t	MT20	5.0	8.0	4.25	2.50
I	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1987.9 lbs FACTORED DOWN AT 1-6-4, 1987.9 lbs FACTORED DOWN AT 3-6-4, 1987.9 lbs FACTORED DOWN AT 5-6-4, 1987.9 lbs FACTORED DOWN AT 7-6-4, 1987.9 lbs FACTORED DOWN AT 9-6-4, AND 1987.9 lbs FACTORED DOWN AT 11-6-4, AND 1992.3 lbs FACTORED DOWN AT 13-6-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	8147	0	8147	0	0	3-8	3-8
I	7367	0	7367	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	6647	3759 / 0	1468 / 0	0 / 0	0 / 0	1420 / 0	0 / 0
I	8010	3400 / 0	1326 / 0	0 / 0	0 / 0	1283 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x6 DRY SPF No.2 T-BRACE AT E-F, D-F

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 8" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-5364 / 0	-84.3	-84.3	0.08 (1)	5.89
B- C	-3573 / 0	-84.3	-84.3	0.06 (1)	6.25
C- D	-3573 / 0	-84.3	-84.3	0.06 (1)	6.25
D- E	-25 / 0	-84.3	-84.3	0.05 (1)	6.25
F- E	-159 / 0	0.0	0.0	0.03 (1)	7.81
I- A	-5856 / 0	0.0	0.0	0.21 (1)	7.20
I- J	0 / 0	-28.0	-28.0	0.27 (1)	10.00
J- K	0 / 0	-28.0	-28.0	0.27 (1)	10.00
K- H	0 / 0	-28.0	-28.0	0.27 (1)	10.00
H- L	0 / 4460	-28.0	-28.0	0.34 (1)	10.00
L- M	0 / 4460	-28.0	-28.0	0.34 (1)	10.00
M- G	0 / 4460	-28.0	-28.0	0.34 (1)	10.00
G- N	0 / 2998	-28.0	-28.0	0.31 (1)	10.00
N- O	0 / 2998	-28.0	-28.0	0.31 (1)	10.00
O- F	0 / 2998	-28.0	-28.0	0.31 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- B	0 / 2609	0.14 (1)
B- G	-2453 / 0	0.54 (1)
G- D	0 / 7179	0.38 (1)
D- F	-6417 / 0	0.82 (1)
A- H	0 / 5200	0.28 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
G	9-6-4	-1988	-1988	—	BACK	VERT	TOTAL
J	1-6-4	-1988	-1988	—	BACK	VERT	TOTAL
K	3-6-4	-1988	-1988	—	BACK	VERT	TOTAL
L	5-6-4	-1988	-1988	—	BACK	VERT	TOTAL
M	7-6-4	-1988	-1988	—	BACK	VERT	TOTAL
N	11-6-4	-1988	-1988	—	BACK	VERT	TOTAL
O	13-6-4	-1992	-1992	—	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.47")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.47")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.21/1.00 (A-I:1) , BC=0.34/1.00 (G-H:1) , WB=0.82/1.00 (D-F:1) , SSI=0.54/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (D) (INPUT = 0.90)
JSI METAL= 0.41 (G) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

BWG NO. TAM 3351-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285929	T90TCX	19	1	TRUSS DESC.	

Temerack Roof Truss, Burlington

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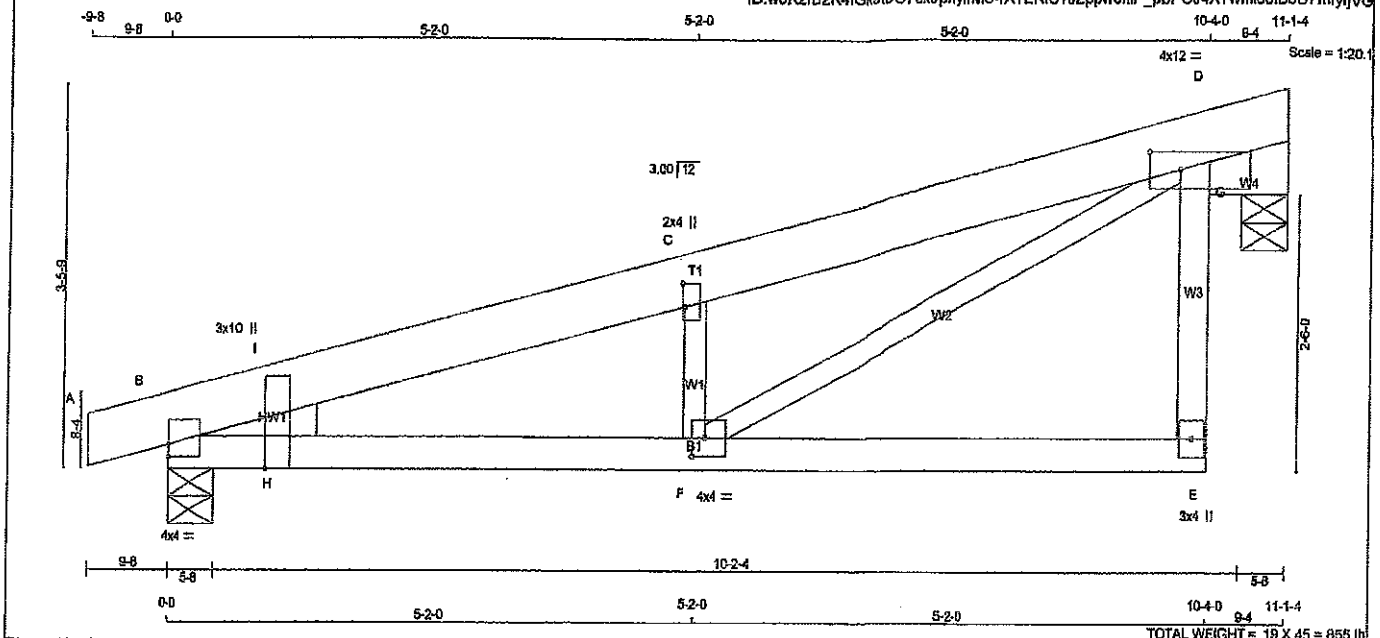

TOTAL WEIGHT = 19 X 45 = 855 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-i	MT20	4.0	4.0	1.25	
B	WP+i	MT20	3.0	10.0	Edge	11.50
C	TMW+w	MT20	2.0	4.0	2.50	0.25
D	TMWW1-p	MT20	4.0	12.0	2.00	3.75
E	BMV+p	MT20	3.0	4.0		
F	BMWW-t	MT20	4.0	4.0	2.00	1.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	RECRD BRG IN-SX	HEEL WEDGE
J(D)	551	0	551	0	0	5-8	5-8	
B	680	0	680	0	0	5-8	5-8	2x4 L

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J(D)	452	251 / 0	103 / 0	0 / 0	0 / 0	98 / 0	0 / 0
B	548	321 / 0	114 / 0	0 / 0	0 / 0	114 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J(D), B

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-4 / 0	-84.3	-84.3	0.02 (1)	10.00
B- I	-1258 / 0	-84.3	-84.3	0.03 (1)	6.25
I- C	-1156 / 0	-84.3	-84.3	0.07 (1)	6.25
C- D	-1152 / 0	-84.3	-84.3	0.17 (1)	6.25
E- G	0 / 106	0.0	0.0	0.34 (1)	10.00
G- D	0 / 106	0.0	0.0	0.34 (1)	10.00
B- H	0 / 1126	-28.0	-28.0	0.20 (1)	10.00
H- F	0 / 1126	-28.0	-28.0	0.28 (2)	10.00
F- E	0 / 103	-28.0	-28.0	0.17 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	-420 / 0	0.09 (1)
F- D	0 / 1157	0.25 (1)
H- I	0 / 131	0.03 (1)
D- J	-765 / 0	0.03 (1)
G- J	0 / 558	0.09 (1)

DWG NO. TAM 45303-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.34 (E-G:1) , BC=0.28 (F-H:2) , WB=0.26 (D-F:1) , SSI=0.15 (D-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (F) (INPUT = 0.90)
JSI METAL= 0.40 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43854	DRWG NO.
285929	G90	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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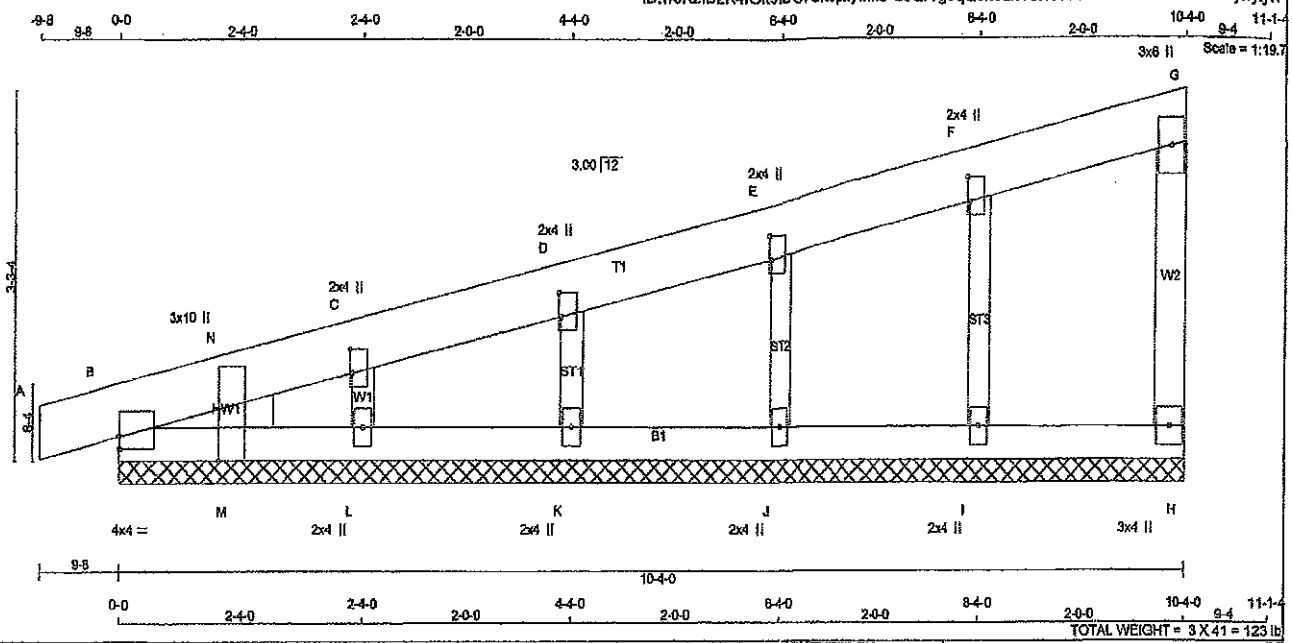

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - G	2x6	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
B - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	4.0	4.0	1.25	
B	WP+I	MT20	3.0	10.0	Edge	11.50
C, D, E, F						
C	TMW+w	MT20	2.0	4.0	2.50	0.25
G	TMV+p	MT20	3.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I, J, K, L						
I	BMW1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

HEEL
WEDGE
2x4 L
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH
A-B	-4 / 0	-84.3	-84.3	0.02 (1)	10.00	
B-N	-46 / 0	-84.3	-84.3	0.02 (1)	6.25	
N-C	-14 / 3	-84.3	-84.3	0.03 (1)	6.25	
C-D	-18 / 0	-84.3	-84.3	0.03 (1)	6.25	
D-E	-9 / 0	-84.3	-84.3	0.02 (1)	10.00	
E-F	-4 / 0	-84.3	-84.3	0.02 (1)	10.00	
F-G	-4 / 0	-84.3	-84.3	0.02 (1)	10.00	
H-G	-72 / 0	0.0	0.0	0.01 (1)	7.61	
B-M	0 / 28	-28.0	-28.0	0.02 (1)	10.00	
M-L	0 / 28	-28.0	-28.0	0.03 (2)	10.00	
L-K	0 / 16	-28.0	-28.0	0.02 (2)	10.00	
K-J	0 / 9	-28.0	-28.0	0.02 (2)	10.00	
J-I	0 / 4	-28.0	-28.0	0.03 (2)	10.00	
I-H	0 / 0	-28.0	-28.0	0.03 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I-F	-168 / 0	0.03 (1)
J-E	-168 / 0	0.03 (1)
K-D	-158 / 0	0.02 (1)
L-C	-201 / 0	0.03 (1)
M-N	0 / 41	0.00 (1)

DWG NO. TAM4553717
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , SBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.03 (C-D:1) , BC=0.03 (I-J:2) , WB=0.03 (F-I:1) , SSI=0.07 (C-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	822	2284	1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (C) (INPUT = 0.60)
JSI METAL= 0.04 (L) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285934	T91	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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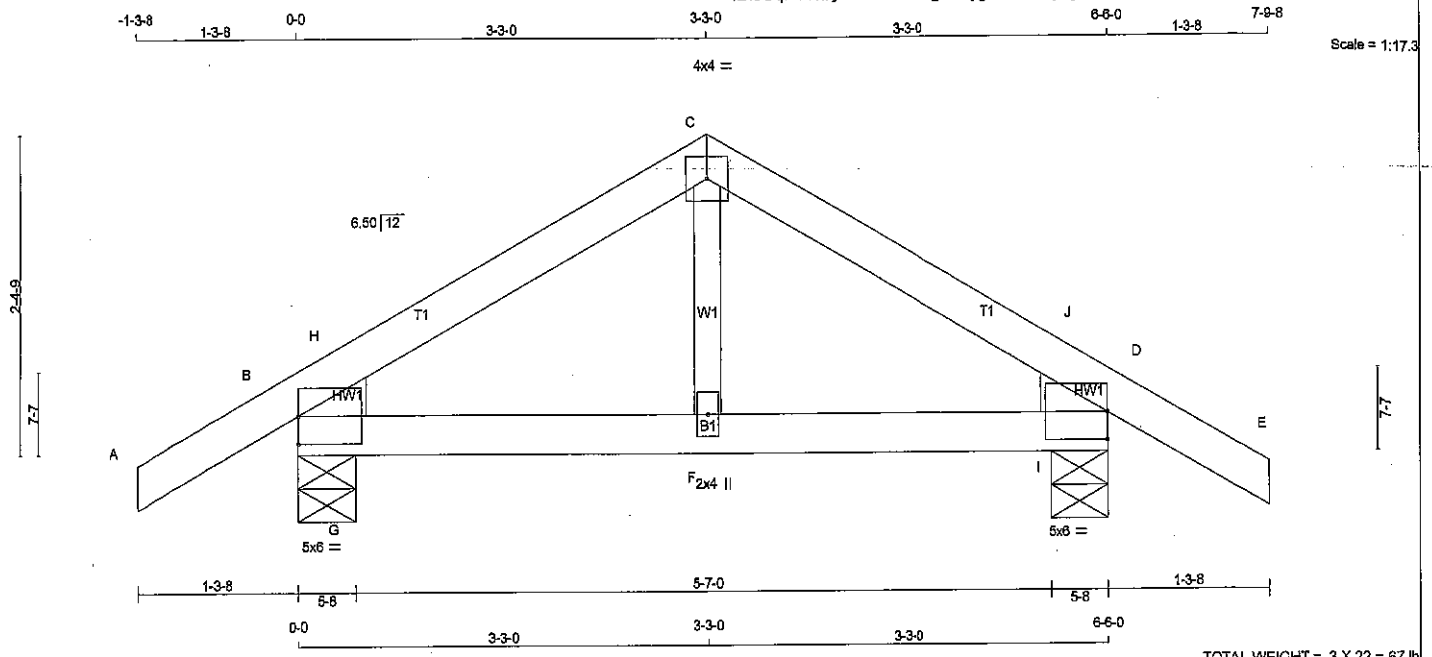


TOTAL WEIGHT = 3 X 22 = 67 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-I	MT20	5.0	6.0	2.50	
C	TTW-p	MT20	4.0	4.0		
D	TMBH1-I	MT20	5.0	6.0	2.50	
F	BMW+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
B	480	0	480	0	0	5-8	5-8	2x4 L
D	480	0	480	0	0	5-8	5-8	2x4 R

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	377	238 / 0	68 / 0	0 / 0	0 / 0	73 / 0	0 / 0
D	377	236 / 0	68 / 0	0 / 0	0 / 0	73 / 0	0 / 0

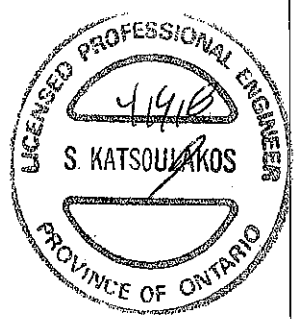
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 17	-84.3	-84.3	0.11 (1)	10.00	F- C	0 / 165	0.04 (2)
B- H	-411 / 0	-84.3	-84.3	0.05 (1)	6.25	G- H	-34 / 113	0.00 (1)
H- C	-355 / 0	-84.3	-84.3	0.08 (1)	6.25	I- J	-33 / 114	0.00 (1)
C- J	-355 / 0	-84.3	-84.3	0.08 (1)	6.25			
J- D	-412 / 0	-84.3	-84.3	0.05 (1)	6.25			
D- E	0 / 17	-84.3	-84.3	0.11 (1)	10.00			
B- G	0 / 305	-28.0	-28.0	0.08 (1)	10.00			
G- F	0 / 305	-28.0	-28.0	0.10 (1)	10.00			
F- I	0 / 305	-28.0	-28.0	0.10 (1)	10.00			
I- D	0 / 305	-28.0	-28.0	0.08 (1)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	48.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.11/1.00 (A-B:1) , BC=0.10/1.00 (F-G:1) , WB=0.04/1.00 (C-F:2) , SSI=0.09/1.00 (C-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (D) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

DWG NO. TAM 21156-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285934	G91	1	1	TRUSS DESC.	

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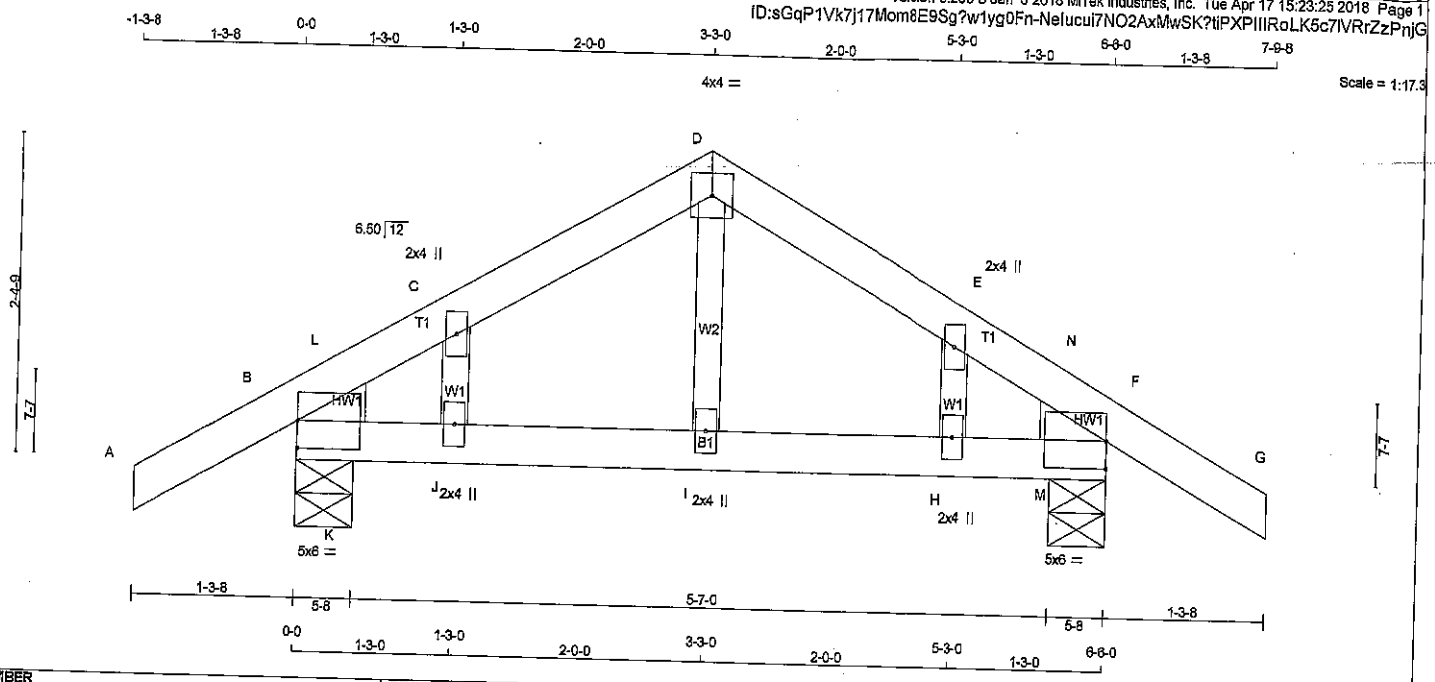

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-I	MT20	5.0	6.0	2.50	
C	TMW+w	MT20	2.0	4.0		
D	TTW-p	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TMBH1-I	MT20	5.0	6.0	2.50	
H, I, J						
H	BMW+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
B	480	0	480	0	0	5-8	5-8	2x4 L
F	480	0	480	0	0	5-8	5-8	2x4 R

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	377	236 / 0	68 / 0	0 / 0	0 / 0	73 / 0	0 / 0
F	377	236 / 0	68 / 0	0 / 0	0 / 0	73 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 17	-84.3	-84.3	0.11 (1)	10.00
B- L	-525 / 0	-84.3	-84.3	0.11 (1)	6.25
L- C	-322 / 0	-84.3	-84.3	0.05 (1)	6.25
C- D	-298 / 0	-84.3	-84.3	0.06 (1)	6.25
D- E	-298 / 0	-84.3	-84.3	0.06 (1)	6.25
E- N	-322 / 0	-84.3	-84.3	0.05 (1)	6.25
N- F	-527 / 0	-84.3	-84.3	0.11 (1)	6.25
F- G	0 / 17	-84.3	-84.3	0.11 (1)	10.00
B- K	0 / 267	-28.0	-28.0	0.11 (1)	10.00
K- J	0 / 267	-28.0	-28.0	0.11 (1)	10.00
J- I	0 / 261	-28.0	-28.0	0.08 (1)	10.00
I- H	0 / 261	-28.0	-28.0	0.08 (1)	10.00
H- M	0 / 267	-28.0	-28.0	0.11 (1)	10.00
M- F	0 / 267	-28.0	-28.0	0.11 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- D	0 / 151	0.03 (2)
H- E	-81 / 26	0.01 (1)
J- C	-81 / 26	0.01 (1)
K- L	0 / 225	0.00 (1)
M- N	0 / 227	0.00 (1)

TOTAL WEIGHT = 23 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.11/1.00 (F-N:1) , BC=0.11/1.00 (H-M:1) , WB=0.03/1.00 (D-I:2) , SSI=0.10/1.00 (F-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.66 (F) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 21157-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285929	T92	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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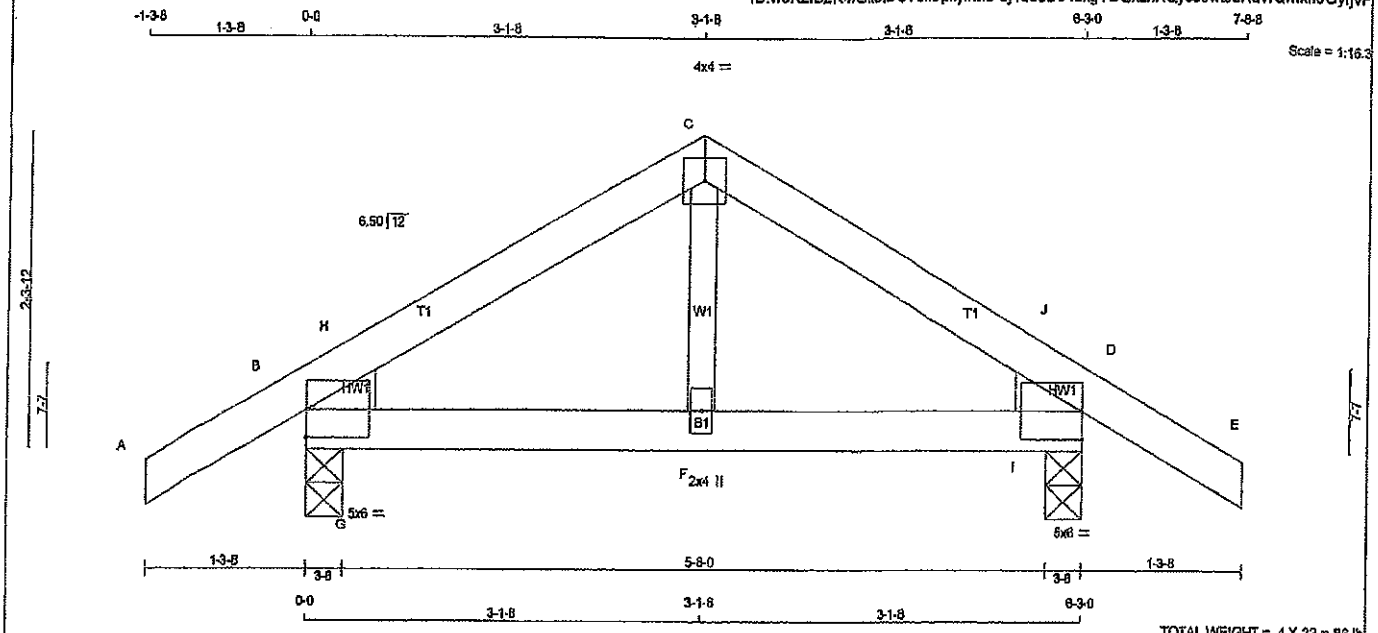

TOTAL WEIGHT = 4 X 22 = 88 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMBH1-I	MT20	5.0	6.0	
C	TTW-p	MT20	4.0	4.0	
D	TMBH1-I	MT20	5.0	6.0	
F	BMW+w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
B	465	0	465	0	0	3-8	3-8	2x4 L
D	465	0	465	0	0	3-8	3-8	2x4 R

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	366	230 / 0	66 / 0	0 / 0	0 / 0	71 / 0	0 / 0
D	366	230 / 0	66 / 0	0 / 0	0 / 0	71 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 17	-84.3	-84.3	0.11 (1)	10.00	
B-H	-397 / 0	-84.3	-84.3	0.04 (1)	6.25	
H-C	-338 / 0	-84.3	-84.3	0.07 (1)	6.25	
C-J	-338 / 0	-84.3	-84.3	0.07 (1)	6.25	
J-D	-397 / 0	-84.3	-84.3	0.04 (1)	6.25	
D-E	0 / 17	-84.3	-84.3	0.11 (1)	10.00	
B-G	0 / 291	-28.0	-28.0	0.08 (1)	10.00	
G-F	0 / 291	-28.0	-28.0	0.09 (1)	10.00	
F-I	0 / 291	-28.0	-28.0	0.09 (1)	10.00	
I-D	0 / 291	-28.0	-28.0	0.08 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F-C	0 / 157	0.04 (2)
G-H	-28 / 107	0.00 (1)
I-J	-28 / 107	0.00 (1)

DWG NO. TAM 45505 17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.21")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.21")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.11 (D-E:1) , BC=0.09 (F-G:1) , WB=0.04 (C-F:2) , SSI=0.09 (C-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (B) (INPUT = 0.90)
JSI METAL= 0.10 (D) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285929	T92Z	2	1	TRUSS DESC.	

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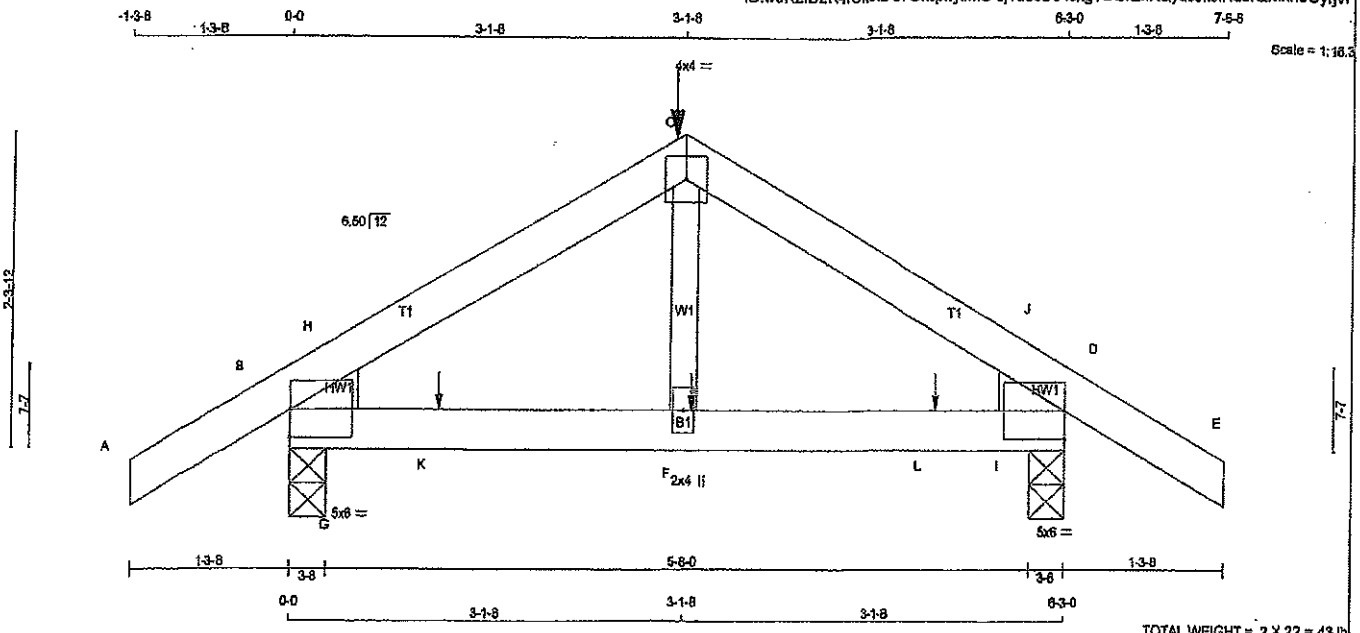

TOTAL WEIGHT = 2 X 22 = 43 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMBH1-I	MT20	5.0	6.0	
C	TTW-p	MT20	4.0	4.0	
D	TMBH1-I	MT20	5.0	6.0	
F	BMW+w	MT20	2.0	4.0	

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 2.7 lbs FACTORED DOWN AT 3-1-8 ON TOP CHORD, AND AT 1-2-4, AND AT 3-2-4, AND AT 5-2-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
B	467	0	467	0	0	3-8	3-8	2x4 L
D	467	0	467	0	0	3-8	3-8	2x4 R

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	367	230 / 0	66 / 0	0 / 0	0 / 0	72 / 0	0 / 0
D	367	230 / 0	66 / 0	0 / 0	0 / 0	72 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 17	-84.3	-84.3	0.12 (1)	10.00	F- C	0 / 157	0.04 (2)
B- H	-399 / 0	-84.3	-84.3	0.08 (1)	6.25	G- H	-28 / 108	0.00 (1)
H- C	-340 / 0	-84.3	-84.3	0.09 (1)	6.25	I- J	-28 / 108	0.00 (1)
C- J	-340 / 0	-84.3	-84.3	0.09 (1)	6.25			
J- D	-399 / 0	-84.3	-84.3	0.08 (1)	6.25			
D- E	0 / 17	-84.3	-84.3	0.12 (1)	10.00			
B- G	0 / 293	-28.0	-28.0	0.08 (1)	10.00			
G- K	0 / 293	-28.0	-28.0	0.10 (1)	10.00			
K- F	0 / 293	-28.0	-28.0	0.10 (1)	10.00			
F- L	0 / 293	-28.0	-28.0	0.10 (1)	10.00			
L- I	0 / 293	-28.0	-28.0	0.10 (1)	10.00			
I- D	0 / 293	-28.0	-28.0	0.08 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	3-1-8	-2	-2	---	BACK	VERT	TOTAL
F	3-2-4	---	---	---	BACK	VERT	TOTAL
K	1-2-4	---	---	---	BACK	VERT	TOTAL
L	5-2-4	---	---	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.21")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/240 (0.21")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.12 (D-E:1) , BC=0.10 (F-G:1) , WB=0.04 (C-F:2) , SSI=0.10 (C-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1687	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (D) (INPUT = 0.90)
JSI METAL= 0.10 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
9117
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 45506-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285929	T130	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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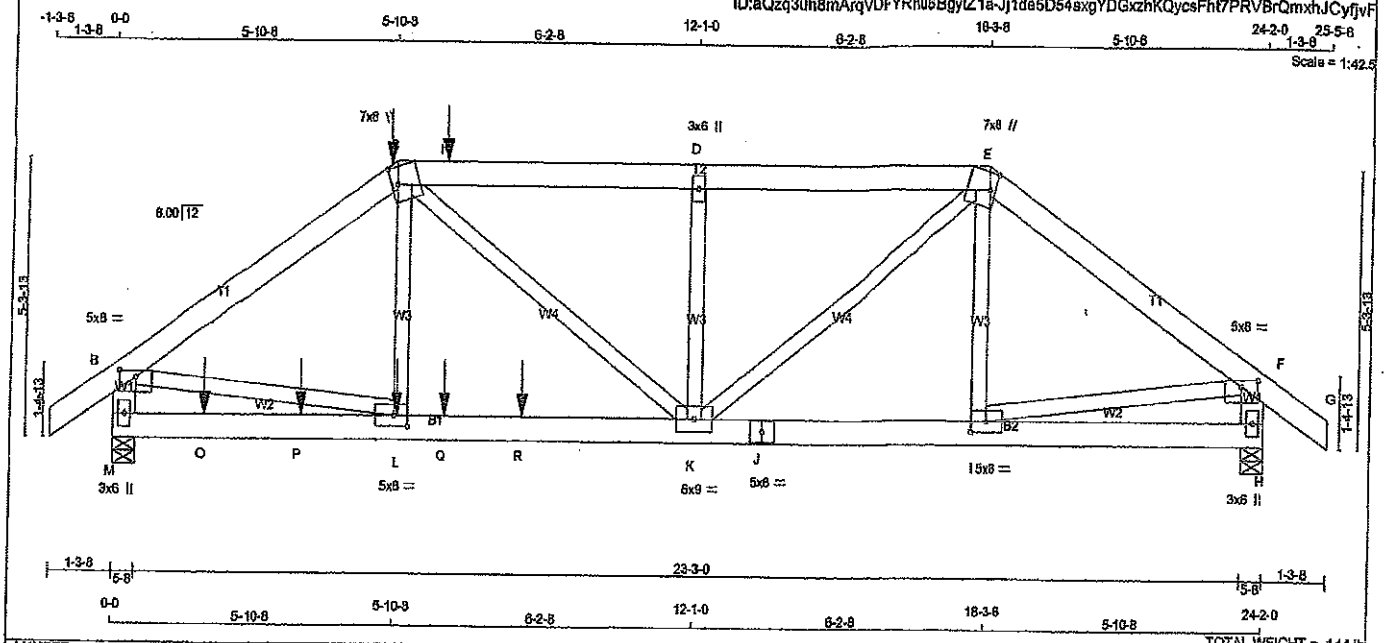

TOTAL WEIGHT = 144 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - G	2x6	DRY	No.2	SPF
M - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
M - J	2x6	DRY	1650F 1.5E	SPF
J - H	2x6	DRY	1650F 1.5E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW-p	MT20	5.0	8.0	1.50	4.00
C	TTWW+m	MT20	7.0	8.0	4.00	1.50
D	TMW+w	MT20	3.0	6.0		
E	TTWW+m	MT20	7.0	8.0	4.00	1.50
F	TMW-p	MT20	5.0	8.0	1.50	4.00
H	BMV1+p	MT20	3.0	6.0		
I	BMWW-t	MT20	5.0	6.0	2.50	3.50
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	6.0	9.0		
L	BMWW-t	MT20	5.0	6.0	2.50	3.50
M	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 474.0 lbs FACTORED DOWN AT 5-10-8, AND 101.4 lbs FACTORED DOWN AT 6-11-4 ON TOP CHORD, AND 87.4 lbs FACTORED DOWN AT 1-11-4, 87.4 lbs FACTORED DOWN AT 3-11-4, 71.7 lbs FACTORED DOWN AT 5-11-4, AND 71.7 lbs FACTORED DOWN AT 6-11-4, AND 1179.5 lbs FACTORED DOWN AT 8-6-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	2817	0	2817	0	0	5-8	5-8
H	2071	0	2071	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	2283	1316 / 0	486 / 0	0 / 0	0 / 0	481 / 0	0 / 0
H	1669	977 / 0	348 / 0	0 / 0	0 / 0	348 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.16 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 33	-84.3	-84.3	0.07 (1)	10.00	L-C	0 / 614	0.14 (2)
B-C	-3566 / 0	-84.3	-84.3	0.36 (1)	4.16	C-K	0 / 463	0.08 (1)
C-N	-3347 / 0	-84.3	-84.3	0.35 (1)	4.28	K-D	-631 / 0	0.17 (1)
N-D	-3347 / 0	-84.3	-84.3	0.35 (1)	4.28	K-E	0 / 1751	0.31 (1)
D-E	-3347 / 0	-84.3	-84.3	0.35 (1)	4.28	I-E	-217 / 141	0.08 (1)
E-F	-2375 / 0	-84.3	-84.3	0.32 (1)	4.97	B-L	0 / 2985	0.53 (1)
F-G	0 / 33	-84.3	-84.3	0.07 (1)	10.00	I-F	0 / 1994	0.35 (1)
M-B	-2794 / 0	0.0	0.0	0.20 (1)	6.21			
H-F	-1983 / 0	0.0	0.0	0.14 (1)	7.13			
M-O	0 / 0	-28.0	-28.0	0.34 (1)	10.00			
O-P	0 / 0	-28.0	-28.0	0.34 (1)	10.00			
P-L	0 / 0	-28.0	-28.0	0.34 (1)	10.00			
L-Q	0 / 2983	-28.0	-28.0	0.89 (1)	10.00			
Q-R	0 / 2983	-28.0	-28.0	0.89 (1)	10.00			
R-K	0 / 2983	-28.0	-28.0	0.89 (1)	10.00			
K-J	0 / 1656	-28.0	-28.0	0.42 (1)	10.00			
J-I	0 / 1656	-28.0	-28.0	0.42 (1)	10.00			
I-H	0 / 0	-28.0	-28.0	0.09 (2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	TYPE
C	5-10-8	-474	-474	---	FRONT	VERT. TOTAL
L	5-11-4	-41	-72	---	FRONT	VERT TOTAL
N	6-11-4	-101	-101	---	FRONT	VERT TOTAL
O	1-11-4	-50	-87	---	FRONT	VERT TOTAL
P	3-11-4	-50	-87	---	FRONT	VERT TOTAL
Q	6-11-4	-41	-72	---	FRONT	VERT TOTAL
R	8-6-8	-1180	-1180	---	FRONT	VERT TOTAL

LICENSED PROFESSIONAL ENGINEER STUART SOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM 45307-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.28")

CSI: TC=0.36 (B-C;1) , BC=0.89 (K-L:1) , WB=0.53 (B-L:1) , SSI=0.55 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (B) (INPUT = 0.90)
JSI METAL= 0.65 (J) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43964	DRWG NO.
285929	T131	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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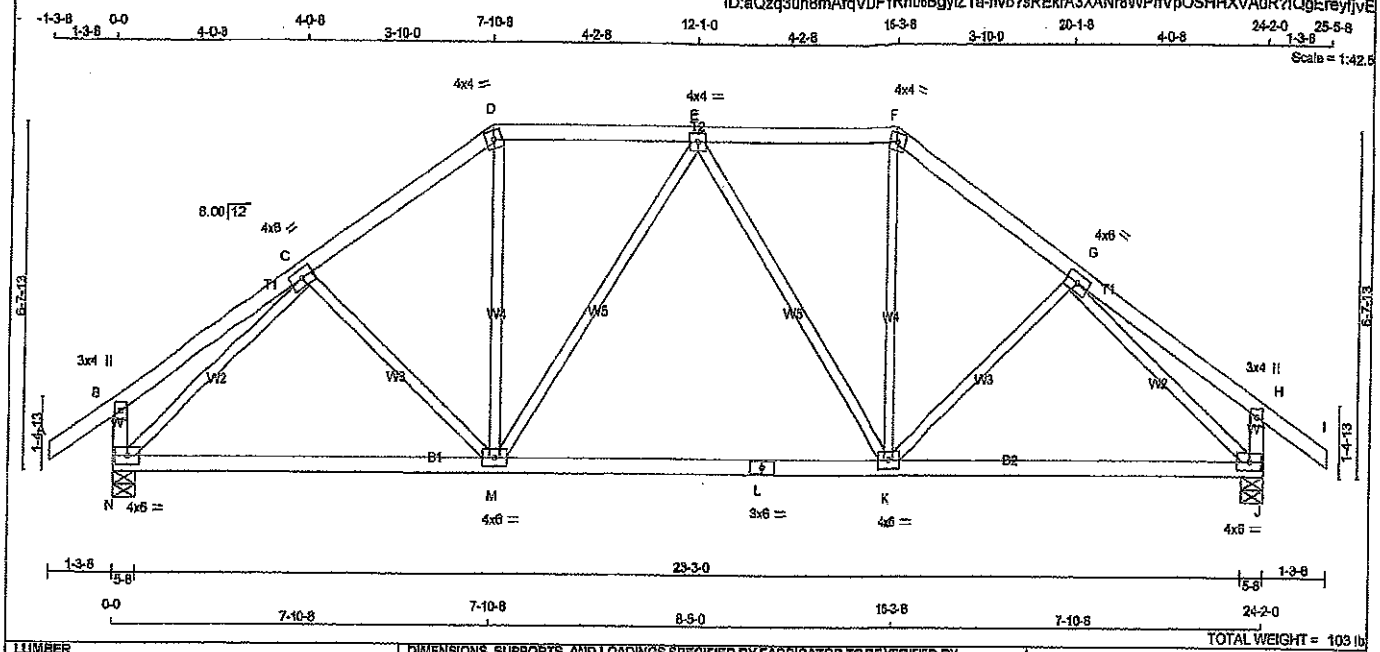

TOTAL WEIGHT = 103 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMV+p	MT20	3.0	4.0	
C	TMWW-t	MT20	4.0	8.0	
D	TTW-m	MT20	4.0	4.0	
E	TMWW-t	MT20	4.0	4.0	
F	TTW-m	MT20	4.0	4.0	
G	TMWW-t	MT20	4.0	6.0	
H	TMV+p	MT20	3.0	4.0	
J	BMVW1-t	MT20	4.0	6.0	
K	BMWWW-t	MT20	4.0	6.0	
L	BS-t	MT20	3.0	6.0	
M	BMWWW-t	MT20	4.0	6.0	
N	BMVW1-t	MT20	4.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1472	0	1472	0	0	5-8	5-8
J	1472	0	1472	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1192	689 / 0	254 / 0	0 / 0	0 / 0	250 / 0	0 / 0
J	1192	689 / 0	254 / 0	0 / 0	0 / 0	250 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.31 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00
B-C	0 / 20	-84.3	-84.3	0.20 (1)	10.00
C-D	-1416 / 0	-84.3	-84.3	0.18 (1)	5.31
D-E	-1168 / 0	-84.3	-84.3	0.20 (1)	5.68
E-F	-1168 / 0	-84.3	-84.3	0.20 (1)	5.68
F-G	-1418 / 0	-84.3	-84.3	0.18 (1)	5.31
G-H	0 / 20	-84.3	-84.3	0.20 (1)	10.00
H-I	0 / 32	-84.3	-84.3	0.11 (1)	10.00
N-B	-245 / 0	0.0	0.0	0.03 (1)	7.81
J-H	-245 / 0	0.0	0.0	0.03 (1)	7.81
N-M	0 / 1208	-28.0	-28.0	0.56 (2)	10.00
M-L	0 / 1306	-28.0	-28.0	0.58 (2)	10.00
L-K	0 / 1306	-28.0	-28.0	0.58 (2)	10.00
K-J	0 / 1208	-28.0	-28.0	0.56 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-M	-65 / 99	0.03 (1)
M-D	0 / 507	0.11 (1)
M-E	-255 / 0	0.29 (1)
E-K	-255 / 0	0.29 (1)
K-F	0 / 507	0.11 (1)
K-G	-65 / 99	0.03 (1)
N-C	-1659 / 0	0.79 (1)
G-J	-1659 / 0	0.79 (1)

DWG NO. TAM 4550817
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/999 (0.26")

CSI: TC=0.20 (D-E:1) , BC=0.58 (K-M:2) , WB=0.79 (C-N:1) , SSI=0.18 (M-N:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (J) (INPUT = 0.90)
JSI METAL= 0.40 (G) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285929	T132	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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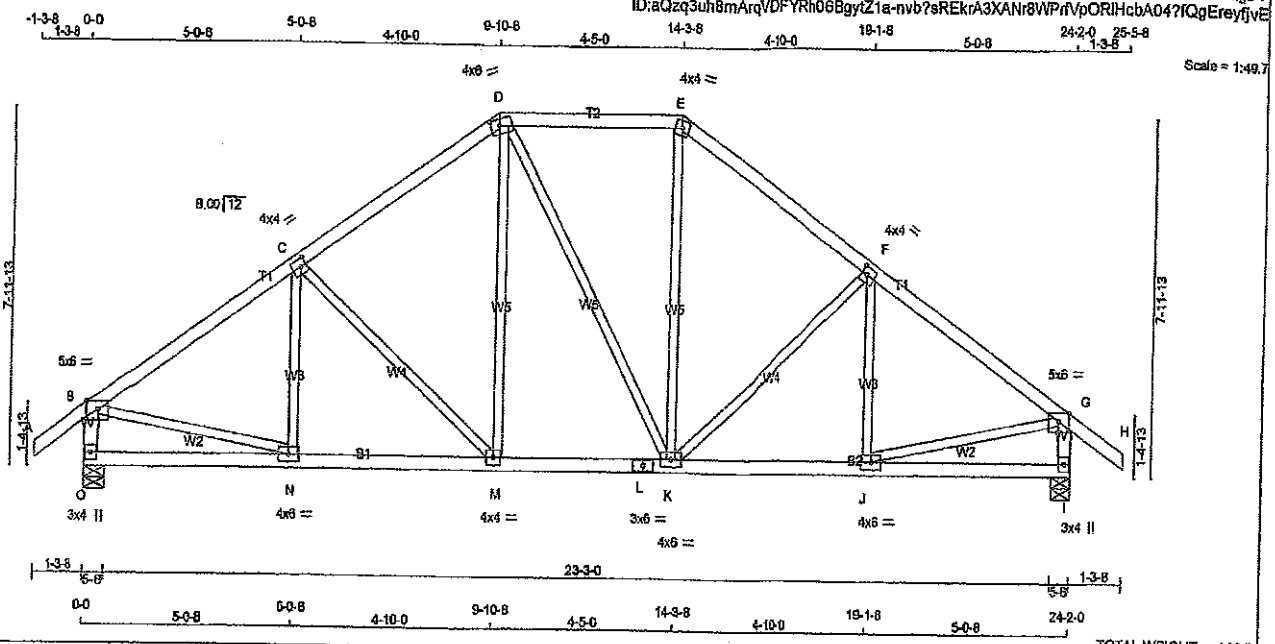


TOTAL WEIGHT = 110 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	4.0	6.0	1.75	2.50
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0	2.00	1.50
G	TMVW-p	MT20	5.0	6.0	Edge	
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMWW-t	MT20	4.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1472	0	1472	0	0	5-8	5-8
I	1472	0	1472	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1192	669 / 0	254 / 0	0 / 0	0 / 0	250 / 0	0 / 0
I	1192	669 / 0	254 / 0	0 / 0	0 / 0	250 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

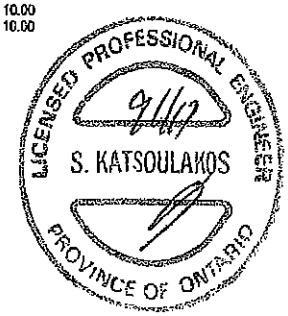
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.03 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	
B-C	-1520 / 0	-84.3	-84.3	0.30 (1)	5.03	
C-D	-1271 / 0	-84.3	-84.3	0.28 (1)	5.40	
D-E	-1039 / 0	-84.3	-84.3	0.22 (1)	5.90	
E-F	-1272 / 0	-84.3	-84.3	0.28 (1)	5.40	
F-G	-1520 / 0	-84.3	-84.3	0.30 (1)	5.03	
G-H	0 / 32	-84.3	-84.3	0.11 (1)	10.00	
O-B	-1414 / 0	0.0	0.0	0.15 (1)	6.86	
I-G	-1413 / 0	0.0	0.0	0.15 (1)	6.86	
O-N	0 / 0	-28.0	-28.0	0.17 (3)	10.00	
N-M	0 / 1287	-28.0	-28.0	0.32 (2)	10.00	
M-L	0 / 1038	-28.0	-28.0	0.23 (2)	10.00	
L-K	0 / 1038	-28.0	-28.0	0.23 (2)	10.00	
K-J	0 / 1287	-28.0	-28.0	0.32 (2)	10.00	
J-I	0 / 0	-28.0	-28.0	0.17 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N-C	-120 / 142	0.04 (1)
M-D	-342 / 0	0.28 (1)
D-K	0 / 374	0.06 (2)
K-E	0 / 2	0.00 (2)
K-F	0 / 376	0.06 (2)
J-F	-340 / 0	0.27 (1)
J-G	-121 / 140	0.04 (1)
B-N	0 / 1316	0.30 (1)
J-G	0 / 1316	0.30 (1)



DWG NO. TAM 45509-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.30 (B-C:1) , BC=0.32 (M-N:2) , WB=0.30 (B-N:1) , SSI=0.17 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	922	2284	1856

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (N) (INPUT = 0.90)
JSI METAL= 0.32 (N) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43654	DRWG NO.
285929	T133	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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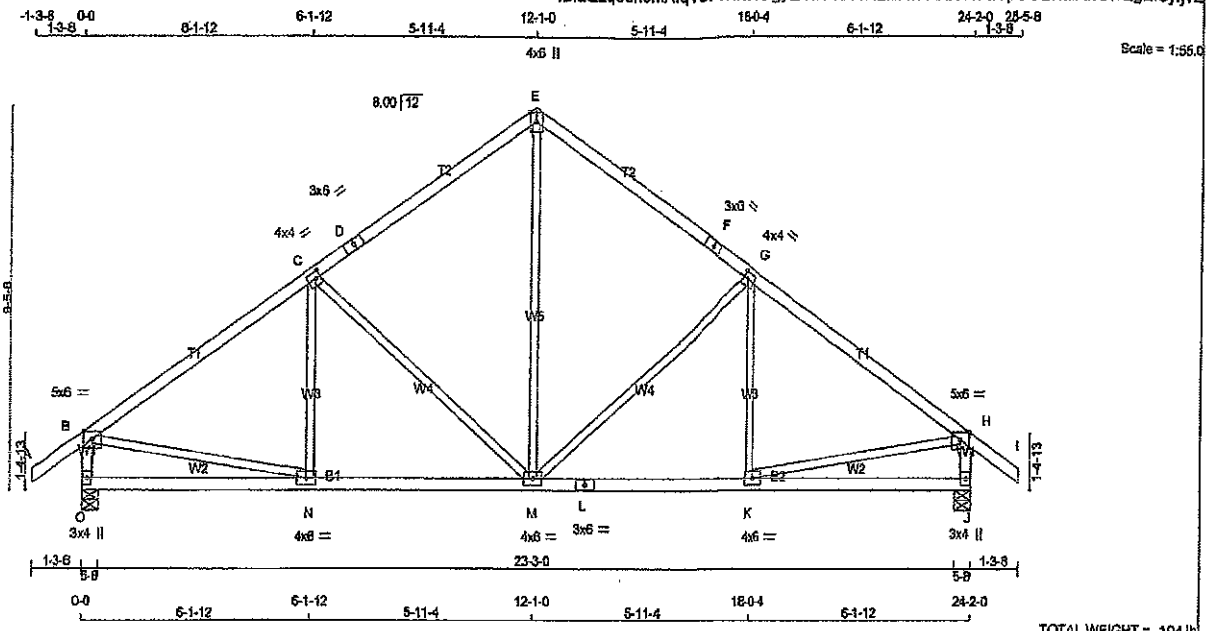

TOTAL WEIGHT = 104 lb

(M)(F)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTW+p	MT20	4.0	6.0	Edge	
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TMWW-p	MT20	5.0	6.0	Edge	
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	4.0	6.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BMWW-t	MT20	4.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1472	0	1472	0	0	5-8	5-8
J	1472	0	1472	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1192	689 / 0	254 / 0	0 / 0	0 / 0	250 / 0	0 / 0
J	1192	689 / 0	254 / 0	0 / 0	0 / 0	250 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.82 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.3	-84.3	0.11 (1)		10.00
B-C	-1519 / 0	-84.3	-84.3	0.46 (1)		4.82
C-D	-1122 / 0	-84.3	-84.3	0.42 (1)		5.46
D-E	-1122 / 0	-84.3	-84.3	0.42 (1)		5.46
E-F	-1122 / 0	-84.3	-84.3	0.42 (1)		5.46
F-G	-1122 / 0	-84.3	-84.3	0.42 (1)		5.46
G-H	-1519 / 0	-84.3	-84.3	0.46 (1)		4.82
H-I	0 / 32	-84.3	-84.3	0.11 (1)		10.00
O-B	-1402 / 0	0.0	0.0	0.14 (1)		8.88
J-H	-1402 / 0	0.0	0.0	0.14 (1)		8.88
O-N	0 / 0	-28.0	-28.0	0.25 (3)		10.00
N-M	0 / 1291	-28.0	-28.0	0.40 (2)		10.00
M-L	0 / 1291	-28.0	-28.0	0.40 (2)		10.00
L-K	0 / 1291	-28.0	-28.0	0.40 (2)		10.00
K-J	0 / 0	-28.0	-28.0	0.25 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M-E	0 / 831	0.19 (1)
M-G	-512 / 0	0.68 (1)
K-G	-38 / 224	0.05 (3)
C-M	-512 / 0	0.68 (1)
N-C	-38 / 224	0.05 (3)
B-N	0 / 1311	0.29 (1)
K-H	0 / 1311	0.29 (1)

DWG NO. T&M 455/0-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.46 (B-C:1) , BC=0.40 (M-N:2) , WB=0.66 (G-M:1) , SSI=0.21 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg

JSI GRIP= 0.85 (N) (INPUT = 0.80)
JSI METAL= 0.35 (L) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285930	T133A	6	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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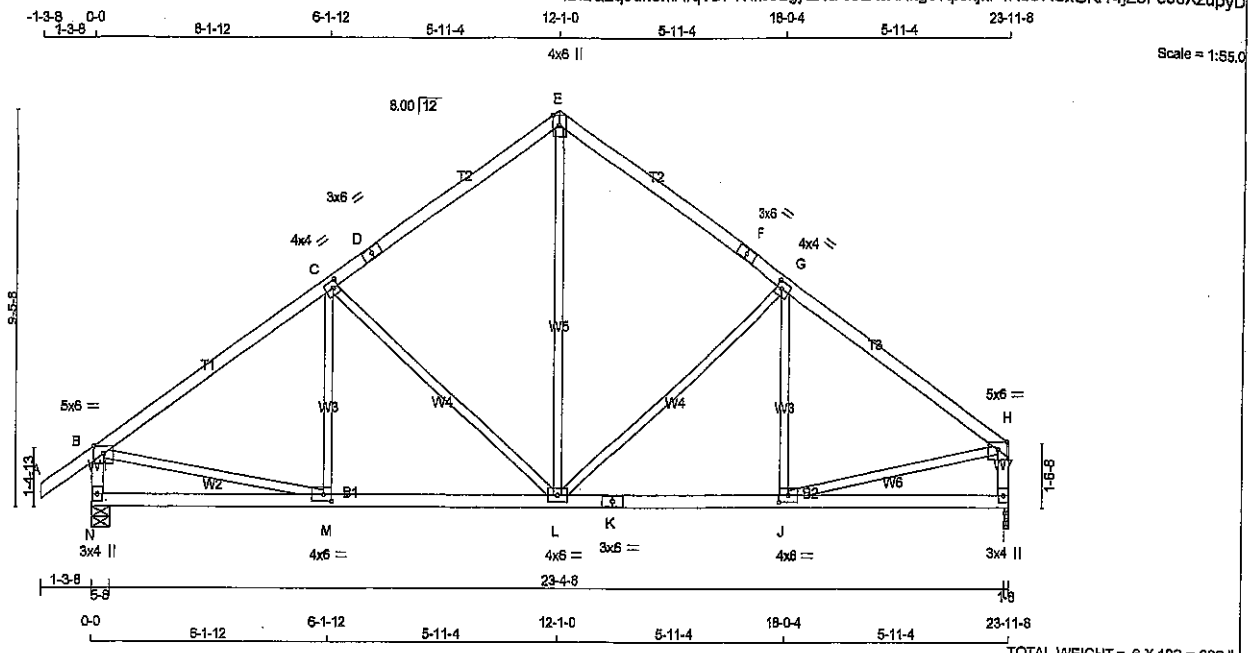


TOTAL WEIGHT = 6 X 102 = 609 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTW+p	MT20	4.0	6.0	Edge	
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW-p	MT20	5.0	6.0	Edge	
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	4.0	6.0	2.00	2.75
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	4.0	6.0		
M	BMWW-t	MT20	4.0	6.0	2.00	2.50
N	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1460	0	1460	0	0	5-8	5-8
I	1345	0	1345	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1183	683 / 0	252 / 0	0 / 0	0 / 0	248 / 0	0 / 0
I	1104	613 / 0	252 / 0	0 / 0	0 / 0	240 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.85 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00
B-C	-1501 / 0	-84.3	-84.3	0.46 (1)	4.85
C-D	-1104 / 0	-84.3	-84.3	0.42 (1)	5.49
D-E	-1104 / 0	-84.3	-84.3	0.42 (1)	5.49
E-F	-1103 / 0	-84.3	-84.3	0.41 (1)	5.52
F-G	-1103 / 0	-84.3	-84.3	0.41 (1)	5.52
G-H	-1465 / 0	-84.3	-84.3	0.43 (1)	4.93
N-B	-1390 / 0	0.0	0.0	0.14 (1)	6.90
I-H	-1277 / 0	0.0	0.0	0.13 (1)	7.13
N-M	0 / 0	-28.0	-28.0	0.25 (3)	10.00
M-L	0 / 1277	-28.0	-28.0	0.40 (2)	10.00
L-K	0 / 1247	-28.0	-28.0	0.38 (2)	10.00
K-J	0 / 1247	-28.0	-28.0	0.38 (2)	10.00
J-I	0 / 0	-28.0	-28.0	0.24 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L-E	0 / 809	0.18 (1)
L-G	-474 / 0	0.61 (1)
J-G	-71 / 202	0.05 (3)
C-L	-514 / 0	0.68 (1)
M-C	-38 / 225	0.05 (3)
B-M	0 / 1297	0.29 (1)
J-H	0 / 1273	0.29 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.60")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.80")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.46/1.00 (B-C:1) , BC=0.40/1.00 (L-M:2) , WB=0.66/1.00 (C-L:1) , SSI=0.21/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

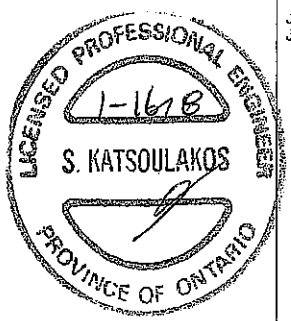
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1867	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (J) (INPUT = 0.90)
JSI METAL= 0.34 (K) (INPUT = 1.00)



DWG NO. TAM 3482-18

STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43934	DRWG NO.
285929	T133C	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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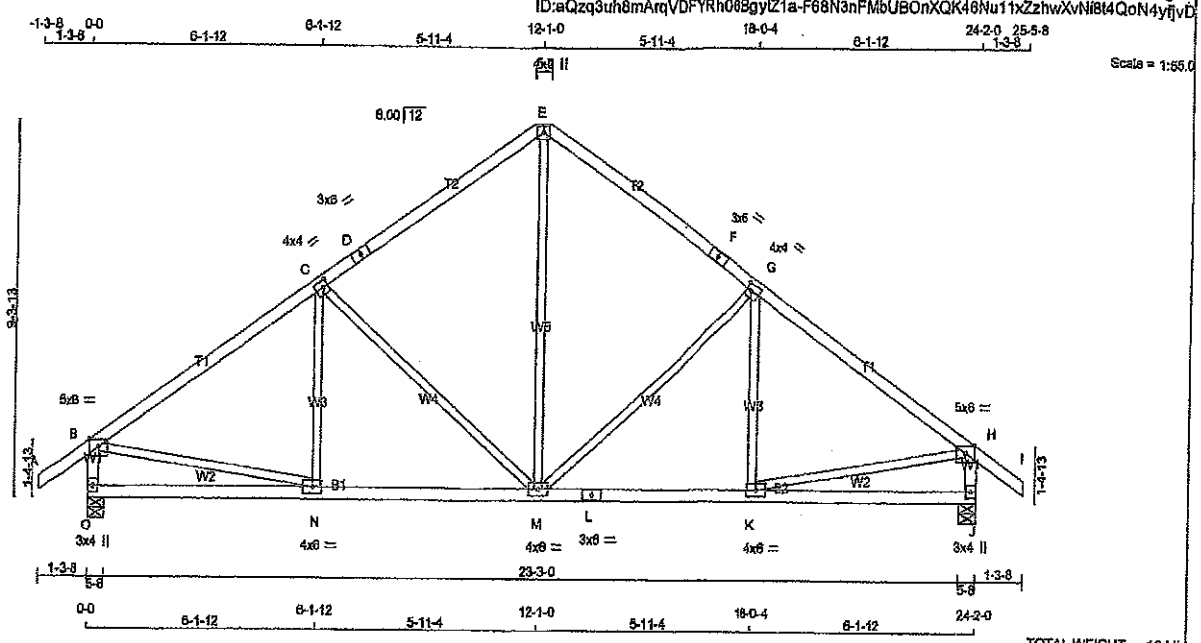


TOTAL WEIGHT = 104 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTW+p	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW-p	MT20	5.0	6.0	Edge	
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	4.0	6.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BMWW-t	MT20	4.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1472	0	1472	0	0	5-8	5-8
J	1472	0	1472	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1192	869 / 0	254 / 0	0 / 0	0 / 0	250 / 0	0 / 0
J	1192	869 / 0	254 / 0	0 / 0	0 / 0	250 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.82 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

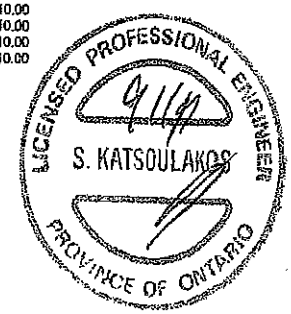
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FR-TO	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF)	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32		-84.3	-84.3	0.11 (1)	10.00	
B-C	-1518 / 0		-84.3	-84.3	0.46 (1)	4.82	
C-D	-1122 / 0		-84.3	-84.3	0.42 (1)	5.46	
D-E	-1122 / 0		-84.3	-84.3	0.42 (1)	5.46	
E-F	-1122 / 0		-84.3	-84.3	0.42 (1)	5.46	
F-G	-1122 / 0		-84.3	-84.3	0.42 (1)	5.46	
G-H	-1518 / 0		-84.3	-84.3	0.46 (1)	4.82	
H-I	0 / 32		-84.3	-84.3	0.11 (1)	10.00	
O-B	-1402 / 0		0.0	0.0	0.14 (1)	6.88	
J-H	-1402 / 0		0.0	0.0	0.14 (1)	6.88	
O-N	0 / 0		-28.0	-28.0	0.25 (3)	10.00	
N-M	0 / 1291		-28.0	-28.0	0.40 (2)	10.00	
M-L	0 / 1291		-28.0	-28.0	0.40 (2)	10.00	
L-K	0 / 1291		-28.0	-28.0	0.40 (2)	10.00	
K-J	0 / 0		-28.0	-28.0	0.25 (3)	10.00	

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M-E	0 / 831	0.19 (1)
M-G	-512 / 0	0.68 (1)
K-G	-38 / 224	0.05 (3)
C-M	-512 / 0	0.68 (1)
N-C	-38 / 224	0.05 (3)
B-N	0 / 1311	0.29 (1)
K-H	0 / 1311	0.29 (1)



DWG NO. YAM 455/12-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	48.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.46 (B-C:1) , BC=0.40 (M-N:2) , WS=0.66 (G-M:1) , SSI=0.21 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	816	354	1697	822	2284	1658

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.65 (N) (INPUT = 0.90)
JSI METAL= 0.35 (L) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285930	T139X	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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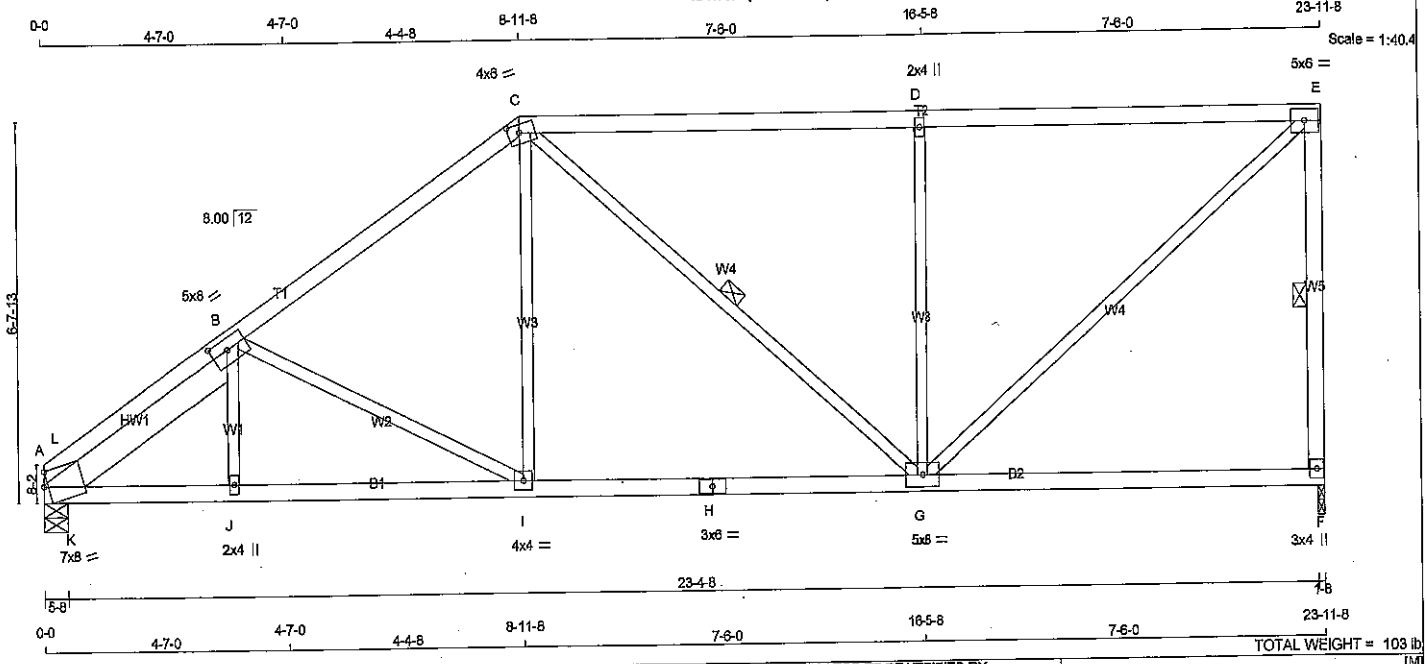

TOTAL WEIGHT = 103 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
A - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
REINFORCING MEMBERS				
HW1	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBMW1-m	MT20	7.0	8.0	3.00	Edge
B	TMWWW-t	MT20	5.0	8.0	2.25	3.50
C	TTWW-m	MT20	4.0	6.0	1.75	2.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	5.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	8.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMW+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
A	1345	0	1345	0	0	5-8	5-8
F	1345	0	1345	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	1104	613 / 0	252 / 0	0 / 0	0 / 0	240 / 0	0 / 0
F	1104	613 / 0	252 / 0	0 / 0	0 / 0	240 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.03 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, C-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- L	-1582 / 0	-84.3	-84.3	0.06 (1)	5.21
L- B	-1097 / 0	-84.3	-84.3	0.31 (1)	5.82
B- C	-1465 / 0	-84.3	-84.3	0.37 (1)	5.01
C- D	-1209 / 0	-84.3	-84.3	0.94 (1)	4.03
D- E	-1209 / 0	-84.3	-84.3	0.94 (1)	4.03
F- E	-1260 / 0	0.0	0.0	0.26 (1)	5.73
A- K	0 / 949	-28.0	-28.0	0.19 (1)	10.00
K- J	0 / 1581	-28.0	-28.0	0.30 (1)	10.00
J- I	0 / 1579	-28.0	-28.0	0.41 (2)	10.00
I- H	0 / 1205	-28.0	-28.0	0.54 (2)	10.00
H- G	0 / 1205	-28.0	-28.0	0.54 (2)	10.00
G- F	0 / 0	-28.0	-28.0	0.40 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- B	0 / 220	0.05 (2)
B- I	-426 / 0	0.25 (1)
I- C	0 / 443	0.10 (2)
C- G	-30 / 5	0.02 (3)
G- D	-784 / 0	0.57 (1)
G- E	0 / 1581	0.36 (1)
K- L	0 / 559	0.00 (1)
K- B	-854 / 0	0.13 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.80")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.80")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.94/1.00 (D-E:1) , BC=0.54/1.00 (G-I:2) , WB=0.57/1.00 (D-G:1) , SSI=0.31/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (G) (INPUT = 0.90)
JSI METAL= 0.47 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
11318
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM3341-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43864	DRWG NO.
285930	T140X	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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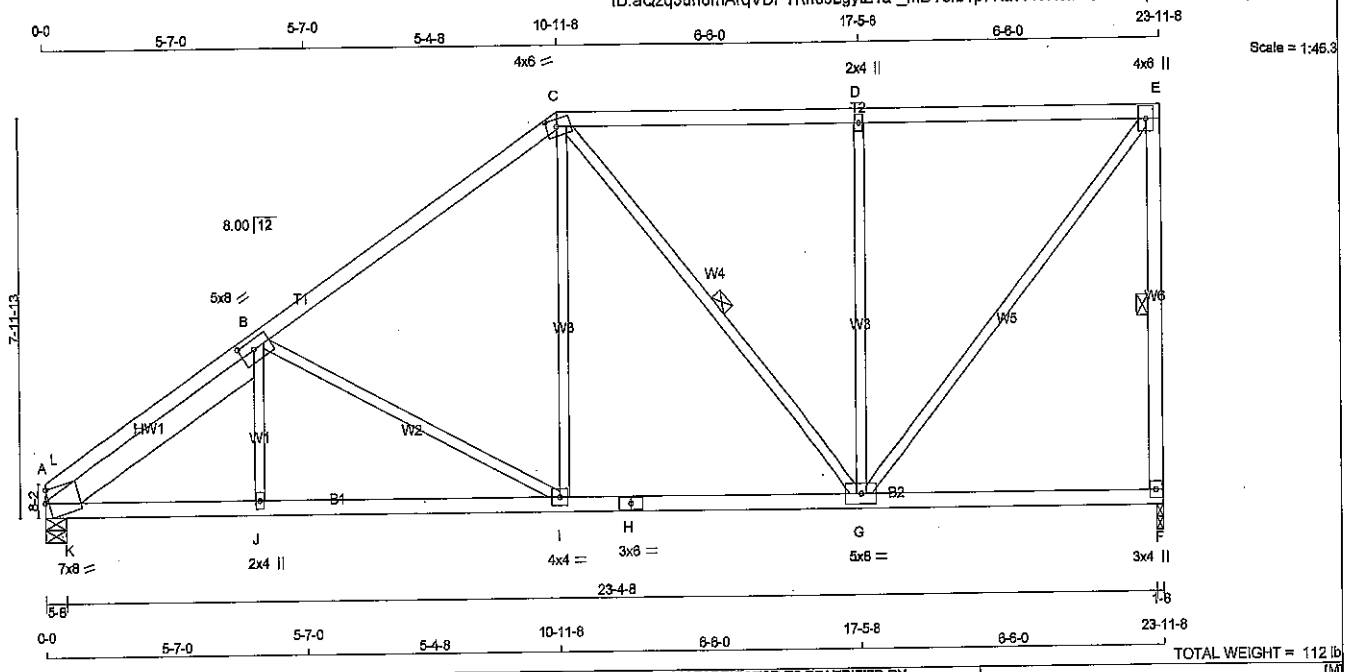


LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
A - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
REINFORCING MEMBERS				
HW1	2x8	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBMW1-m	MT20	7.0	8.0	3.00	Edge
B	TMWWW-t	MT20	5.0	8.0	2.25	3.50
C	TTWW-m	MT20	4.0	6.0	1.75	2.50
D	TMW+w	MT20	2.0	4.0		
E	TMW+p	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	8.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMW+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
A	1345	0	1345	0	0	5-8	5-8
F	1345	0	1345	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	1104	613 / 0	252 / 0	0 / 0	0 / 0	240 / 0	0 / 0
F	1104	613 / 0	252 / 0	0 / 0	0 / 0	240 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.03 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-F, C-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

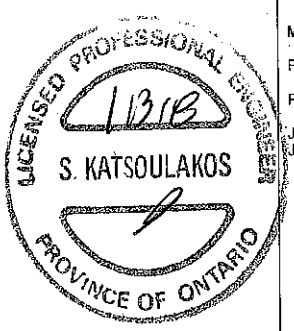
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- L	-1609 / 0	-84.3	-84.3	0.04 (1)	5.20
L- B	-1135 / 0	-84.3	-84.3	0.46 (1)	5.29
B- C	-1269 / 0	-84.3	-84.3	0.54 (1)	5.03
C- D	-899 / 0	-84.3	-84.3	0.65 (1)	5.38
D- E	-899 / 0	-84.3	-84.3	0.65 (1)	5.38
F- E	-1271 / 0	0.0	0.0	0.36 (1)	5.71
A- K	0 / 980	-28.0	-28.0	0.19 (1)	10.00
K- J	0 / 1570	-28.0	-28.0	0.37 (2)	10.00
J- I	0 / 1568	-28.0	-28.0	0.41 (2)	10.00
I- H	0 / 1055	-28.0	-28.0	0.41 (2)	10.00
H- G	0 / 1055	-28.0	-28.0	0.41 (2)	10.00
G- F	0 / 0	-28.0	-28.0	0.29 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- B	0 / 286	0.06 (2)
B- I	-589 / 0	0.62 (1)
I- C	0 / 506	0.11 (2)
C- G	-241 / 0	0.17 (1)
G- D	-679 / 0	0.82 (1)
G- E	0 / 1393	0.31 (1)
K- L	0 / 506	0.00 (1)
K- B	-774 / 0	0.19 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.80")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.80")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.65/1.00 (D-E:1) , BC=0.41/1.00 (G-I:2) , WB=0.82/1.00 (D-G:1) , SSI=0.27/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

USI GRIP= 0.88 (E) (INPUT = 0.90)
USI METAL= 0.32 (A) (INPUT = 1.00)

DWG NO. TAM 33612-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285930	T141X	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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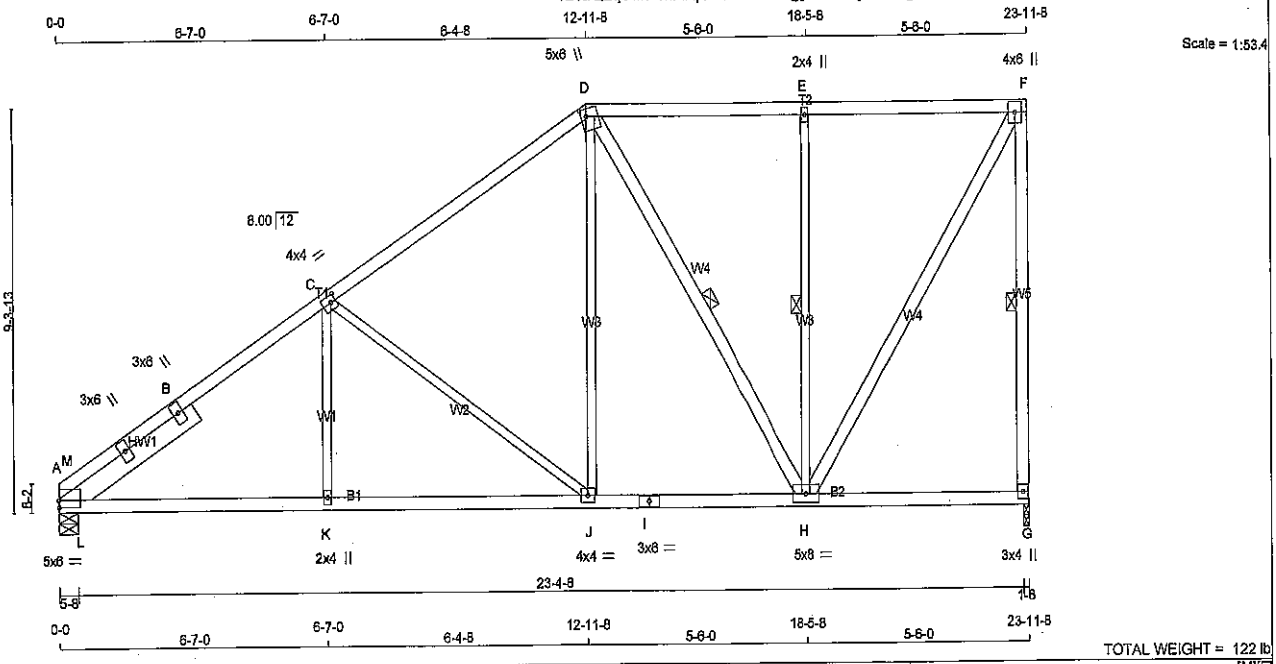


TOTAL WEIGHT = 122 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
A - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
REINFORCING MEMBERS				
HW1	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBMR1-I	MT20	5.0	6.0	2.00	0.25
A	RT+I	MT20	3.0	6.0		
A	RT+t	MT20	3.0	6.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMWW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMW+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
A	1345	0	1345	0	0	5-8	5-8
G	1345	0	1345	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	1104	613 / 0	252 / 0	0 / 0	0 / 0	240 / 0	0 / 0
G	1104	613 / 0	252 / 0	0 / 0	0 / 0	240 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.96 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX. CSI	MAX. UNBRAC LENGTH
A- M	-1708 / 0	-84.3	-84.3	0.06 (1)		5.06
M- B	-1480 / 0	-84.3	-84.3	0.40 (1)		4.96
B- C	-1480 / 0	-84.3	-84.3	0.40 (1)		4.96
C- D	-1100 / 0	-84.3	-84.3	0.43 (1)		5.48
D- E	-877 / 0	-84.3	-84.3	0.33 (1)		8.25
E- F	-678 / 0	-84.3	-84.3	0.33 (1)		8.25
G- F	-1281 / 0	0.0	0.0	0.52 (1)		5.69
A- L	0 / 1074	-28.0	-28.0	0.26 (2)		10.00
L- K	0 / 1444	-28.0	-28.0	0.41 (2)		10.00
K- J	0 / 1444	-28.0	-28.0	0.41 (2)		10.00
J- I	0 / 897	-28.0	-28.0	0.30 (2)		10.00
I- H	0 / 897	-28.0	-28.0	0.30 (2)		10.00
H- G	0 / 0	-28.0	-28.0	0.20 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	0 / 326	0.07 (2)
C- J	-667 / 0	0.93 (1)
J- D	0 / 570	0.13 (2)
D- H	-423 / 0	0.25 (1)
H- E	-574 / 0	0.33 (1)
H- F	0 / 1295	0.21 (1)
L- M	0 / 469	0.00 (1)
L- B	-515 / 0	0.06 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.80")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.80")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.52/1.00 (F-G:1) , BC=0.41/1.00 (K-L:2) , WB=0.93/1.00 (C-J:1) , SSI=0.23/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

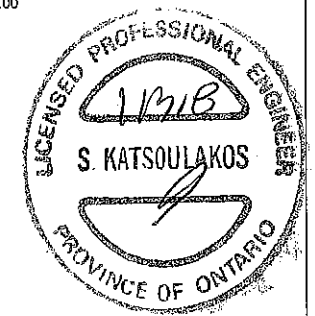
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.72 (F) (INPUT = 0.90)
JSI METAL= 0.43 (A) (INPUT = 1.00)



DWG NO. TAM 3343-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285929	T142	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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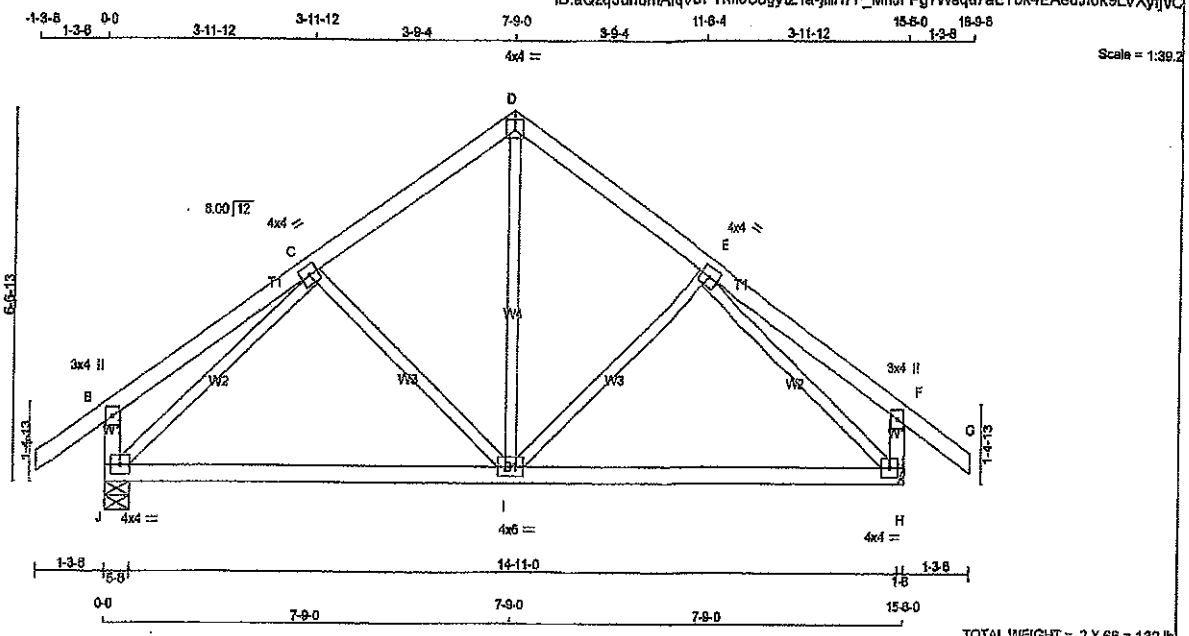

TOTAL WEIGHT = 2 X 66 = 132 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER.

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
J	986	0	986	0	0	IN-SX	IN-SX
H	986	0	986	0	0	5-8	5-8
						HANGER BY OTHERS MIN. SEAT SIZE: I-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	793	467 / 0	163 / 0	0 / 0	0 / 0	163 / 0	0 / 0
H	793	467 / 0	163 / 0	0 / 0	0 / 0	163 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	
B- C	0 / 21	-84.3	-84.3	0.20 (1)	10.00	
C- D	-708 / 0	-84.3	-84.3	0.16 (1)	6.25	
D- E	-708 / 0	-84.3	-84.3	0.16 (1)	6.25	
E- F	0 / 21	-84.3	-84.3	0.20 (1)	10.00	
F- G	0 / 32	-84.3	-84.3	0.11 (1)	10.00	
J- B	-241 / 0	0.0	0.0	0.02 (1)	7.81	
H- F	-241 / 0	0.0	0.0	0.02 (1)	7.81	
J- I	0 / 700	-28.0	-28.0	0.57 (2)	10.00	
I- H	0 / 700	-28.0	-28.0	0.57 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- D	0 / 509	0.11 (1)
I- E	-175 / 43	0.09 (1)
C- I	-175 / 43	0.09 (1)
J- C	-963 / 0	0.44 (1)
E- H	-963 / 0	0.44 (1)

DWG NO. TAM 45516 17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	9.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.52")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.52")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.20 (E-F:1) , BC=0.57 (H-I:2) , WB=0.44 (E-H:1) , SSI=0.18 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (D) (INPUT = 0.90)
JSI METAL= 0.34 (C) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285929	G142	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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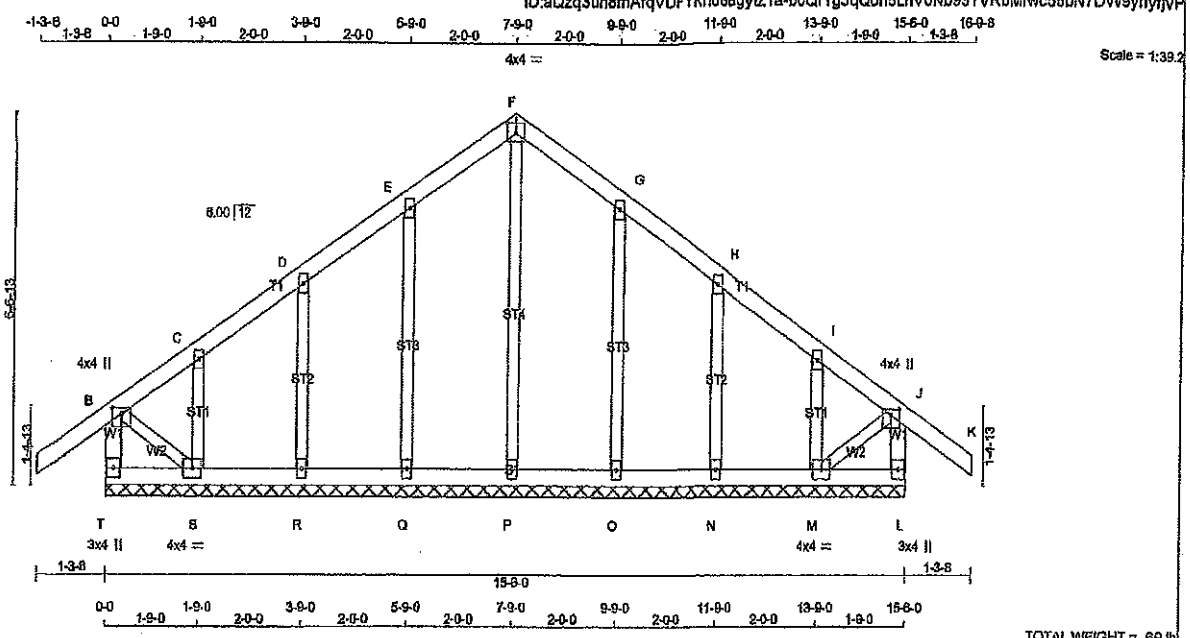

TOTAL WEIGHT = 69 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - F	2x4	DRY	No.2	SPF
F - K	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C, D, E, G, H, I						
C	TMW+w	MT20	2.0	4.0		
F	TTW-p	MT20	4.0	4.0	2.25	2.00
J	TMVW+p	MT20	4.0	4.0	1.25	2.00
L	BMV1+p	MT20	3.0	4.0		
M	BMWW1-t	MT20	4.0	4.0		
N, O, P, Q, R						
N	BMW1+w	MT20	2.0	4.0		
S	BMWW1-t	MT20	4.0	4.0		
T	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	P-F	-129 / 0	0.09 (1)
B-C	-47 / 0	-84.3	-84.3	0.11 (1)	6.25	Q-E	-169 / 0	0.08 (1)
C-D	-14 / 0	-84.3	-84.3	0.04 (1)	6.25	R-D	-173 / 0	0.04 (1)
D-E	-13 / 0	-84.3	-84.3	0.05 (1)	6.25	S-C	-102 / 0	0.02 (1)
E-F	-20 / 0	-84.3	-84.3	0.05 (1)	6.25	O-G	-169 / 0	0.08 (1)
F-G	-20 / 0	-84.3	-84.3	0.05 (1)	6.25	N-H	-173 / 0	0.04 (1)
G-H	-13 / 0	-84.3	-84.3	0.05 (1)	6.25	M-I	-102 / 0	0.02 (1)
H-I	-14 / 0	-84.3	-84.3	0.04 (1)	6.25	B-S	0 / 23	0.01 (1)
I-J	-47 / 0	-84.3	-84.3	0.11 (1)	6.25	M-J	0 / 23	0.01 (1)
J-K	0 / 32	-84.3	-84.3	0.11 (1)	10.00			
T-B	-252 / 0	0.0	0.0	0.09 (1)	7.81			
L-J	-252 / 0	0.0	0.0	0.03 (1)	7.81			
T-S	0 / 0	-28.0	-28.0	0.02 (2)	10.00			
S-R	0 / 16	-28.0	-28.0	0.02 (2)	10.00			
R-Q	0 / 12	-28.0	-28.0	0.02 (2)	10.00			
Q-P	0 / 9	-28.0	-28.0	0.02 (2)	10.00			
P-O	0 / 9	-28.0	-28.0	0.02 (2)	10.00			
O-N	0 / 12	-28.0	-28.0	0.02 (2)	10.00			
N-M	0 / 16	-28.0	-28.0	0.02 (2)	10.00			
M-L	0 / 0	-28.0	-28.0	0.02 (2)	10.00			

LICENSED PROFESSIONAL ENGINEER
9147
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 45539-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11 (A-B:1) , BC=0.02 (M-N:2) , WB=0.09 (F-P:1) , SSI=0.08 (A-B,1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1687	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.16 (J) (INPUT = 0.90)
JSI METAL= 0.05 (E) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285929	T144	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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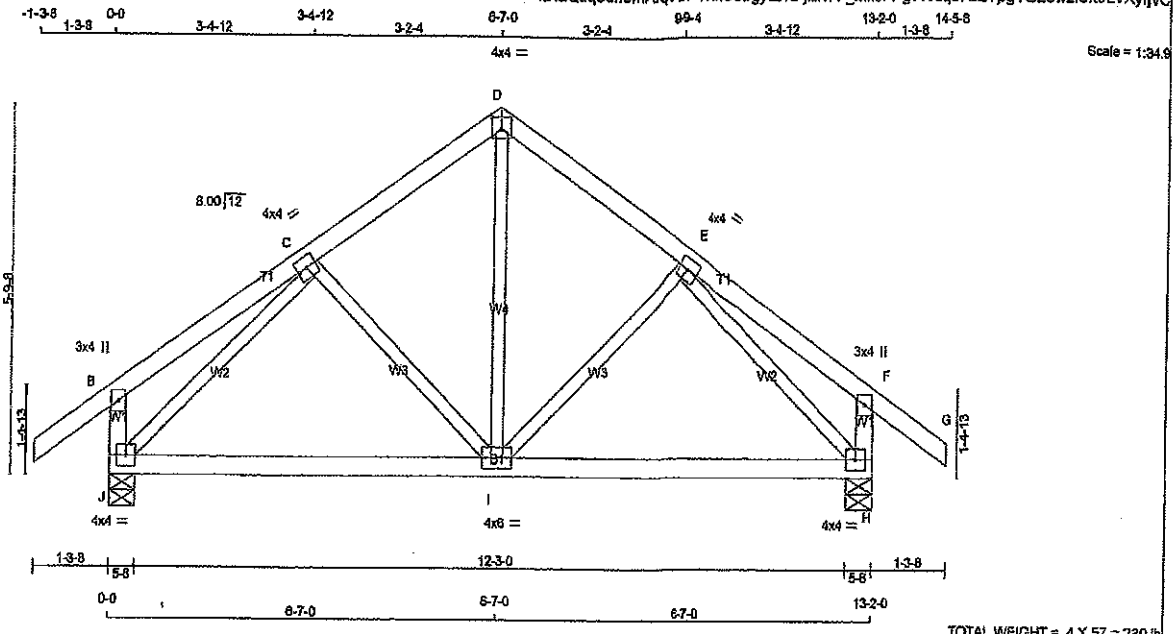

TOTAL WEIGHT = 4 X 57 = 230 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	8.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	855	0	855	0	0	5-8	5-8
H	855	0	855	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	665	407 / 0	138 / 0	0 / 0	0 / 0	140 / 0	0 / 0
H	665	407 / 0	138 / 0	0 / 0	0 / 0	140 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00
B-C	0 / 17	-84.3	-84.3	0.14 (1)	10.00
C-D	-583 / 0	-84.3	-84.3	0.11 (1)	6.25
D-E	-583 / 0	-84.3	-84.3	0.11 (1)	6.25
E-F	0 / 17	-84.3	-84.3	0.14 (1)	10.00
F-G	0 / 32	-84.3	-84.3	0.11 (1)	10.00
J-B	-223 / 0	0.0	0.0	0.02 (1)	7.81
H-F	-223 / 0	0.0	0.0	0.02 (1)	7.81
J-I	0 / 563	-28.0	-28.0	0.41 (2)	10.00
I-H	0 / 563	-28.0	-28.0	0.41 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I-D	0 / 411	0.09 (1)
I-E	-130 / 44	0.05 (1)
C-I	-130 / 44	0.05 (1)
J-C	-795 / 0	0.27 (1)
E-H	-795 / 0	0.27 (1)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 45517-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , SCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.44")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.44")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.14 (E-F:1) , BC=0.41 (I-J:2) , WB=0.27 (E-H:1) , SSI=0.16 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.70 (C) (INPUT = 0.90)
JSI METAL= 0.28 (E) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285929	T185	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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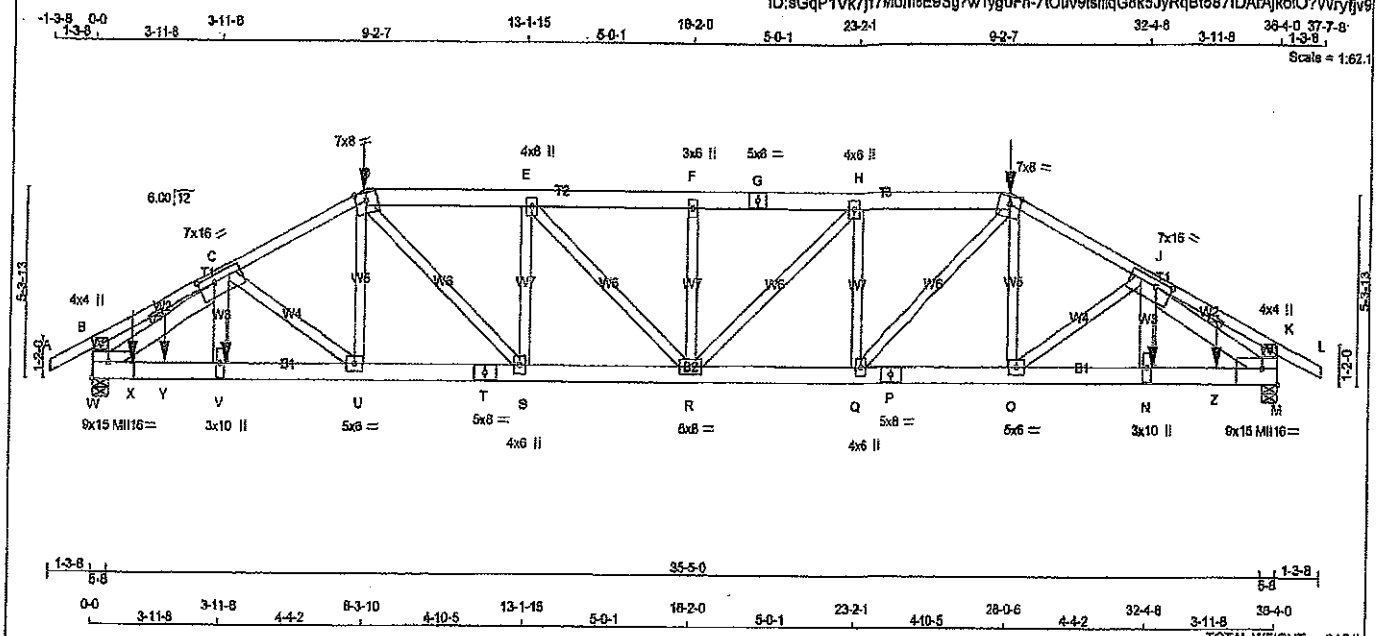

TOTAL WEIGHT = 216 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x6	DRY	No.2	SPF
G - I	2x6	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
W - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
W - T	2x6	DRY	1650F 1.5E	SPF
T - P	2x6	DRY	1650F 1.5E	SPF
P - M	2x6	DRY	1650F 1.5E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
V - C	2x6	DRY	No.2	SPF
N - J	2x6	DRY	No.2	SPF
W - C	2x6	DRY	No.2	SPF
J - M	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWWW-t	MT20	7.0	16.0	2.25	6.00
D	TTWW-m	MT20	7.0	8.0	Edge	
E	TMWW+t	MT20	4.0	6.0		
F	TMW+w	MT20	3.0	6.0		
G	TS-t	MT20	5.0	6.0		
H	TMWW+t	MT20	4.0	6.0		
I	TTWW-m	MT20	7.0	8.0	Edge	
J	TMWWW-t	MT20	7.0	16.0	2.25	6.00
K	TMV+p	MT20	4.0	4.0		
M	BMVW1-t	MII16	9.0	15.0	5.50	Edge
N	BMW+w	MT20	3.0	10.0		
O	BMWW-t	MT20	5.0	6.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWW+t	MT20	4.0	6.0		
R	BMWWW-t	MT20	5.0	8.0		
S	BMWW+t	MT20	4.0	6.0		
T	BS-t	MT20	5.0	8.0		
U	BMWW-t	MT20	5.0	6.0		
V	BMW+w	MT20	3.0	10.0		
W	BMVW1-t	MII16	9.0	15.0	5.50	Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 499.5 lbs FACTORED DOWN AT 6-3-10, AND 499.5 lbs FACTORED DOWN AT 28-0-6 ON TOP CHORD, AND 71.7 lbs FACTORED DOWN AT 1-2-4, 71.7 lbs FACTORED DOWN AT 2-2-4, 2338.0 lbs FACTORED DOWN AT 4-1-6, AND 2338.1 lbs FACTORED DOWN AT 32-5-8, AND 71.7 lbs FACTORED DOWN AT 34-5-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
W	5071	0	5071	0	0	5-8	5-8
M	5034	0	5034	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	4084	2397 / 0	863 / 0	0 / 0	0 / 0	844 / 0	0 / 0
M	4037	2397 / 0	815 / 0	0 / 0	0 / 0	825 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.09 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-W, J-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 26	-84.3	-84.3	0.12 (1)	10.00	V-C	0 / 2354	0.31 (1)
B-C	-2 / 13	-84.3	-84.3	0.17 (1)	10.00	C-U	-1452 / 0	0.42 (1)
C-D	-8115 / 0	-84.3	-84.3	0.89 (1)	2.09	U-D	0 / 1014	0.18 (1)
D-E	-6338 / 0	-84.3	-84.3	0.40 (1)	3.20	O-I	0 / 1008	0.18 (1)
E-F	-6608 / 0	-84.3	-84.3	0.40 (1)	3.14	O-J	-1434 / 0	0.41 (1)
F-G	-6608 / 0	-84.3	-84.3	0.40 (1)	3.14	N-J	0 / 2332	0.30 (1)
G-H	-6606 / 0	-84.3	-84.3	0.40 (1)	3.14	W-C	-8156 / 0	0.72 (1)
H-I	-6332 / 0	-84.3	-84.3	0.40 (1)	3.20	J-M	-8130 / 0	0.72 (1)
I-J	-6108 / 0	-84.3	-84.3	0.89 (1)	2.09	Q-I	0 / 1266	0.22 (1)
J-K	-2 / 13	-84.3	-84.3	0.17 (1)	10.00	D-S	0 / 1282	0.22 (1)
K-L	0 / 28	-84.3	-84.3	0.12 (1)	10.00	Q-H	-767 / 0	0.21 (1)
W-B	-253 / 0	0.0	0.0	0.02 (1)	7.81	S-E	-763 / 0	0.21 (1)
M-K	-253 / 0	0.0	0.0	0.02 (1)	7.81	R-H	0 / 486	0.07 (1)
						E-R	0 / 1286	0.22 (1)
W-X	0 / 6697	-28.0	-28.0	0.68 (1)	10.00	R-S	0 / 1281	0.19 (1)
X-Y	0 / 6697	-28.0	-28.0	0.68 (1)	10.00			
Y-V	0 / 6697	-28.0	-28.0	0.68 (1)	10.00			
V-U	0 / 6688	-28.0	-28.0	0.67 (1)	10.00			
U-T	0 / 5448	-28.0	-28.0	0.48 (1)	10.00			
T-S	0 / 5448	-28.0	-28.0	0.48 (1)	10.00			
S-R	0 / 6335	-28.0	-28.0	0.56 (1)	10.00			
R-O	0 / 6332	-28.0	-28.0	0.56 (1)	10.00			
Q-P	0 / 5439	-28.0	-28.0	0.48 (1)	10.00			
P-O	0 / 5439	-28.0	-28.0	0.48 (1)	10.00			
O-N	0 / 6687	-28.0	-28.0	0.67 (1)	10.00			
N-Z	0 / 6678	-28.0	-28.0	0.67 (1)	10.00			
Z-M	0 / 6676	-28.0	-28.0	0.67 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
D	6-3-10	-45	-50	---	FRONT	VERT	DEAD
D	6-3-10	-455	-455	---	FRONT	VERT	SNOW
I	28-0-6	-45	-50	---	FRONT	VERT	DEAD
I	28-0-6	-455	-455	---	FRONT	VERT	SNOW
N	32-6-8	-2338	-2338	---	TOP	VERT.	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.21")
CALCULATED VERT. DEFL.(LL) = L/ 959 (0.34")
ALLOWABLE DEFL.(TL)= L/360 (1.21")
CALCULATED VERT. DEFL.(TL) = L/ 803 (0.54")

CSI: TC=0.89 (C-D:1) , BC=0.68 (V-W:1) , WB=0.72 (C-W:1) , SSI=0.16 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656
MII16	473	276	2341	1245	4454	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (M) (INPUT = 0.90)
JSI METAL= 1.00 (J) (INPUT = 1.00)

CONTINUED ON PAGE 2

LICENSED PROFESSIONAL ENGINEER S. KATSOULAKOS PROVINCE OF ONTARIO 9161

DWG NO. TAM45518-17

STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285929	T185	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
V	4-1-8	-2338	-2338	—	FRONT	VERT	TOTAL
X	1-2-4	-41	-72	—	FRONT	VERT	TOTAL
Y	2-2-4	-41	-72	—	FRONT	VERT	TOTAL
Z	34-5-12	-41	-72	—	FRONT	VERT	TOTAL

DWG NO. TAM 455/18-17 Rev 2
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43554	DRWG NO.
285929	T186	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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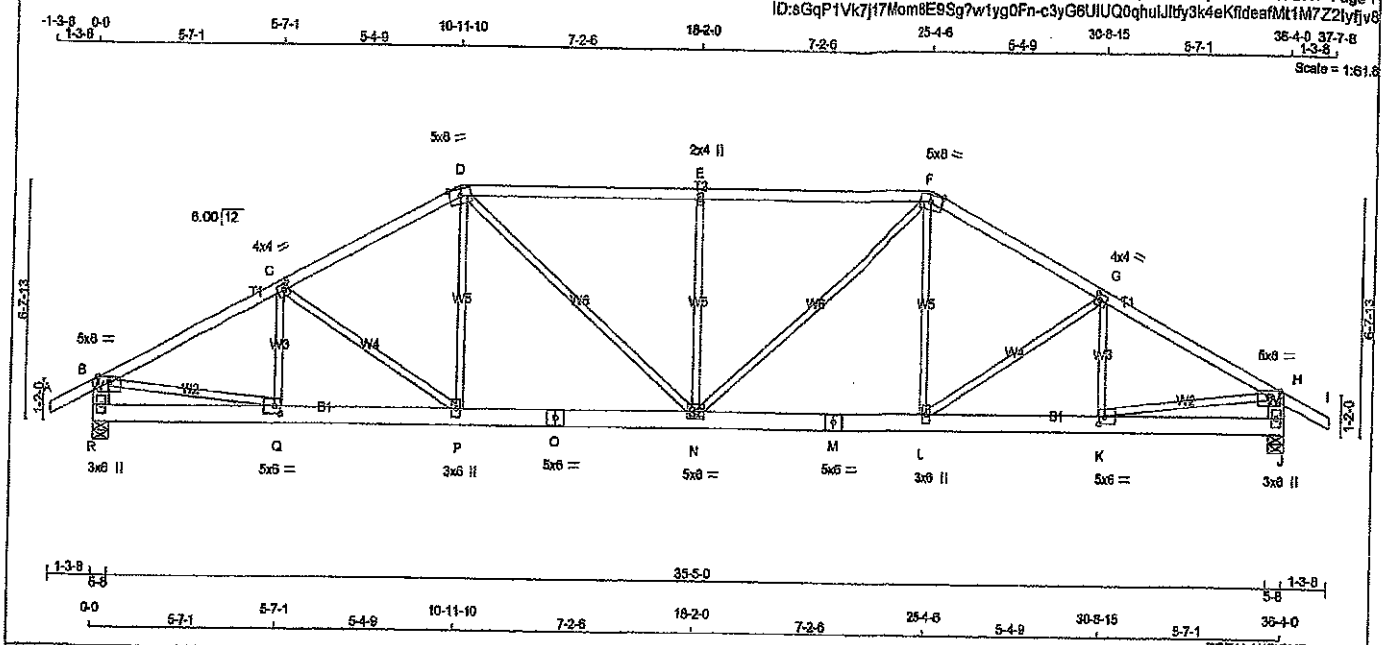

TOTAL WEIGHT = 168 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
O - M	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	8.0	Edge	
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	8.0	Edge	
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
H	TMVW-p	MT20	5.0	8.0	Edge	
J	BMV1+p	MT20	3.0	8.0		
K	BMWW-t	MT20	5.0	6.0	2.50	2.00
L	SMWW+t	MT20	3.0	6.0		
M	BS-t	MT20	5.0	6.0		
N	BMWWW-t	MT20	5.0	6.0		
O	BS-t	MT20	5.0	6.0		
P	BMWW+t	MT20	3.0	6.0		
Q	BMWW-t	MT20	5.0	6.0	2.50	2.00
R	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2154	0	2154	0	0	5-8	5-8
J	2154	0	2154	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1752	999 / 0	382 / 0	0 / 0	0 / 0	371 / 0	0 / 0
J	1752	999 / 0	382 / 0	0 / 0	0 / 0	371 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	VERT. LOAD LC1 FROM	TO	MAX CSI (LC)
A-B	0 / 28	-84.3	-84.3	0.11 (1)
B-C	-3076 / 0	-84.3	-84.3	0.48 (1)
C-D	-2827 / 0	-84.3	-84.3	0.44 (1)
D-E	-3045 / 0	-84.3	-84.3	0.78 (1)
E-F	-3045 / 0	-84.3	-84.3	0.78 (1)
F-G	-2827 / 0	-84.3	-84.3	0.44 (1)
G-H	-3076 / 0	-84.3	-84.3	0.48 (1)
H-I	0 / 28	-84.3	-84.3	0.11 (1)
R-B	-2068 / 0	0.0	0.0	0.13 (1)
J-H	-2068 / 0	0.0	0.0	0.13 (1)
R-Q	0 / 0	-28.0	-28.0	0.11 (2)
Q-P	0 / 2772	-28.0	-28.0	0.40 (1)
P-O	0 / 2512	-28.0	-28.0	0.37 (1)
O-N	0 / 2512	-28.0	-28.0	0.37 (1)
N-M	0 / 2512	-28.0	-28.0	0.37 (1)
M-L	0 / 2512	-28.0	-28.0	0.37 (1)
L-K	0 / 2772	-28.0	-28.0	0.40 (1)
K-J	0 / 0	-28.0	-28.0	0.11 (2)

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B-C	-284 / 60	0.06 (1)
C-P	-319 / 0	0.24 (1)
P-D	0 / 442	0.10 (2)
D-N	0 / 707	0.16 (1)
N-E	-746 / 0	0.51 (1)
N-F	0 / 707	0.16 (1)
L-F	0 / 442	0.10 (2)
L-G	-319 / 0	0.24 (1)
K-G	-284 / 60	0.06 (1)
B-Q	0 / 2799	0.63 (1)
K-H	0 / 2799	0.63 (1)

MAX. UNBRAC LENGTH	FR-TO
10.00	
3.59	
3.77	
3.20	
3.20	
3.77	
3.59	
10.00	
7.05	
7.05	
10.00	
10.00	
10.00	
10.00	
10.00	
10.00	
10.00	
10.00	

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. T AM45519 -17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.21")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.20")
ALLOWABLE DEFL.(TL)= L/360 (1.21")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.31")

CSI: TC=0.78 (E-F:1) , BC=0.40 (P-Q:1) , WB=0.63 (B-Q:1) , SSI=0.28 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1567	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.69 (K) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43964	DRWG NO.
285929	T187	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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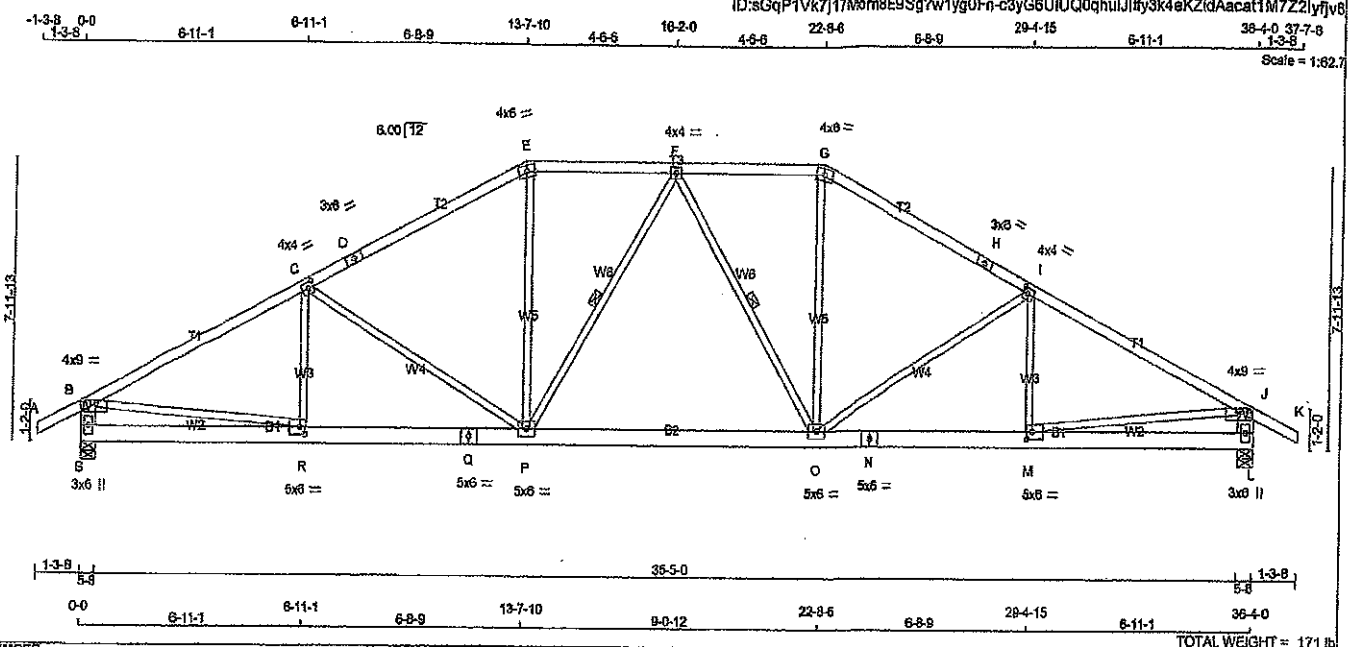


TOTAL WEIGHT = 171 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
S - Q	2x6	DRY	No.2	SPF
Q - N	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	9.0	1.00	4.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	6.0		
E	TTW-m	MT20	4.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTW-m	MT20	4.0	6.0		
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	4.0	4.0	2.00	1.75
J	TMVW-p	MT20	4.0	9.0	1.00	4.75
L	BMV1+p	MT20	3.0	6.0		
M	BMWW-t	MT20	5.0	6.0	2.50	2.00
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	6.0		
P	BMWWW-t	MT20	5.0	6.0		
Q	BS-t	MT20	5.0	6.0		
R	BMWW-t	MT20	5.0	6.0	2.50	2.00
S	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	2154	0	2154	0	0	5-8	5-8
L	2154	0	2154	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1752	999 / 0	392 / 0	0 / 0	0 / 0	371 / 0	0 / 0
L	1752	999 / 0	392 / 0	0 / 0	0 / 0	371 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.26 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

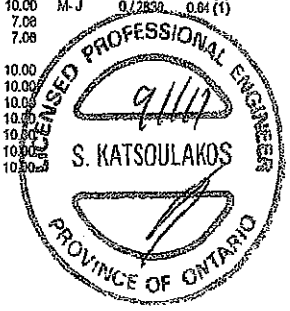
1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-P, F-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 26	-84.3	-84.3	0.11 (1)	10.00	R-C	-139 / 189	0.04 (3)	
B-C	-3115 / 0	-84.3	-84.3	0.79 (1)	3.26	C-P	-804 / 0	0.81 (1)	
C-D	-2603 / 0	-84.3	-84.3	0.66 (1)	3.63	P-E	0 / 788	0.18 (1)	
D-E	-2603 / 0	-84.3	-84.3	0.66 (1)	3.63	P-F	-287 / 0	0.14 (1)	
E-F	-2310 / 0	-84.3	-84.3	0.28 (1)	4.27	F-O	-287 / 0	0.14 (1)	
F-G	-2310 / 0	-84.3	-84.3	0.28 (1)	4.27	O-G	0 / 788	0.18 (1)	
G-H	-2603 / 0	-84.3	-84.3	0.66 (1)	3.63	O-I	-804 / 0	0.81 (1)	
H-I	-2603 / 0	-84.3	-84.3	0.66 (1)	3.63	M-I	-139 / 189	0.04 (3)	
I-J	-3115 / 0	-84.3	-84.3	0.79 (1)	3.26	B-R	0 / 2830	0.64 (1)	
J-K	0 / 26	-84.3	-84.3	0.11 (1)	10.00	M-J	0 / 2830	0.64 (1)	
S-B	-2060 / 0	0.0	0.0	0.13 (1)	7.08				
L-J	-2060 / 0	0.0	0.0	0.13 (1)	7.08				
S-R	0 / 0	-28.0	-28.0	0.15 (2)	10.00				
R-Q	0 / 2613	-28.0	-28.0	0.43 (1)	10.00				
Q-P	0 / 2813	-28.0	-28.0	0.43 (1)	10.00				
P-O	0 / 2444	-28.0	-28.0	0.39 (1)	10.00				
O-N	0 / 2813	-28.0	-28.0	0.43 (1)	10.00				
N-M	0 / 2813	-28.0	-28.0	0.43 (1)	10.00				
M-L	0 / 0	-28.0	-28.0	0.15 (2)	10.00				



DWG NO. TAM 45520-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.21")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.19")
ALLOWABLE DEFL.(TL)= L/360 (1.21")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.31")

CSI: TC=0.79 (B-C:1) , BC=0.43 (P-R:1) , WB=0.81 (C-P:1) , SSI=0.25 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (R) (INPUT = 0.90)
JSI METAL= 0.70 (R) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285929	T188	12	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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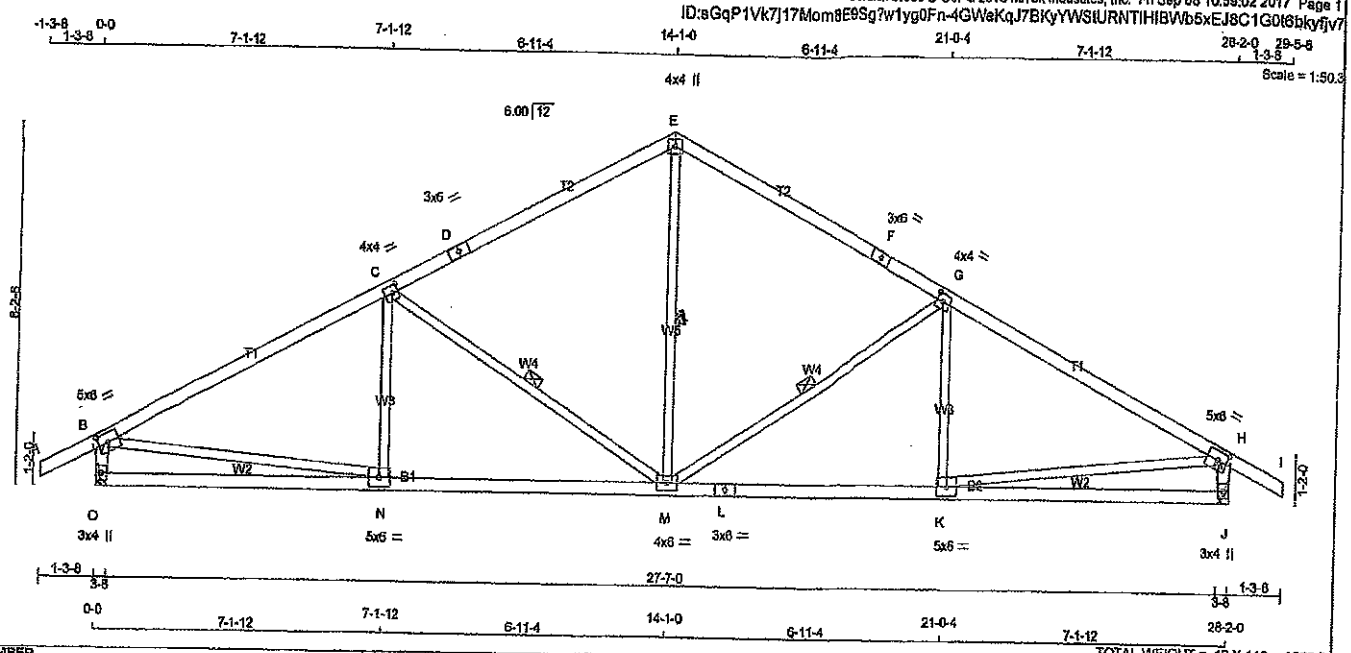

TOTAL WEIGHT = 12 X 110 = 1322 lb (M)[F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	6.0		
E	TTW+p	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
H	TMVW-t	MT20	5.0	6.0	2.50	2.75
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1695	0	1695	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	
J	1695	0	1695	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1376	790 / 0	286 / 0	0 / 0	0 / 0	290 / 0	0 / 0
J	1376	790 / 0	286 / 0	0 / 0	0 / 0	290 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.60 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-M, C-M. E-M

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 26	-84.3	-84.3	0.11 (1)	10.00	
B-C	-2254 / 0	-84.3	-84.3	0.70 (1)	3.60	
C-D	-1622 / 0	-84.3	-84.3	0.61 (1)	4.45	
D-E	-1622 / 0	-84.3	-84.3	0.61 (1)	4.45	
E-F	-1622 / 0	-84.3	-84.3	0.61 (1)	4.45	
F-G	-1622 / 0	-84.3	-84.3	0.61 (1)	4.45	
G-H	-2254 / 0	-84.3	-84.3	0.70 (1)	3.89	
H-I	0 / 26	-84.3	-84.3	0.11 (1)	10.00	
O-B	-1613 / 0	0.0	0.0	0.18 (1)	6.52	
J-H	-1613 / 0	0.0	0.0	0.18 (1)	6.52	
O-N	0 / 0	-28.0	-28.0	0.34 (3)	10.00	
N-M	0 / 2044	-28.0	-28.0	0.57 (2)	10.00	
M-L	0 / 2044	-28.0	-28.0	0.57 (2)	10.00	
L-K	0 / 2044	-28.0	-28.0	0.57 (2)	10.00	
K-J	0 / 0	-28.0	-28.0	0.34 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M-E	0 / 692	0.22 (1)
M-G	-738 / 0	0.34 (1)
K-G	-27 / 289	0.06 (3)
C-M	-738 / 0	0.34 (1)
N-C	-27 / 289	0.06 (3)
B-N	0 / 2059	0.46 (1)
K-H	0 / 2059	0.46 (1)

LICENSED PROFESSIONAL ENGINEER
9142
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 45524-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NECC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TFIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.94")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.94")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.70 (B-C:1) , BC=0.57 (M-N:2) , WB=0.46 (B-N:1) , SSI=0.26 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1697	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (K) (INPUT = 0.90)
JSI METAL= 0.61 (H) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43854	DRWG NO.
285929	T189A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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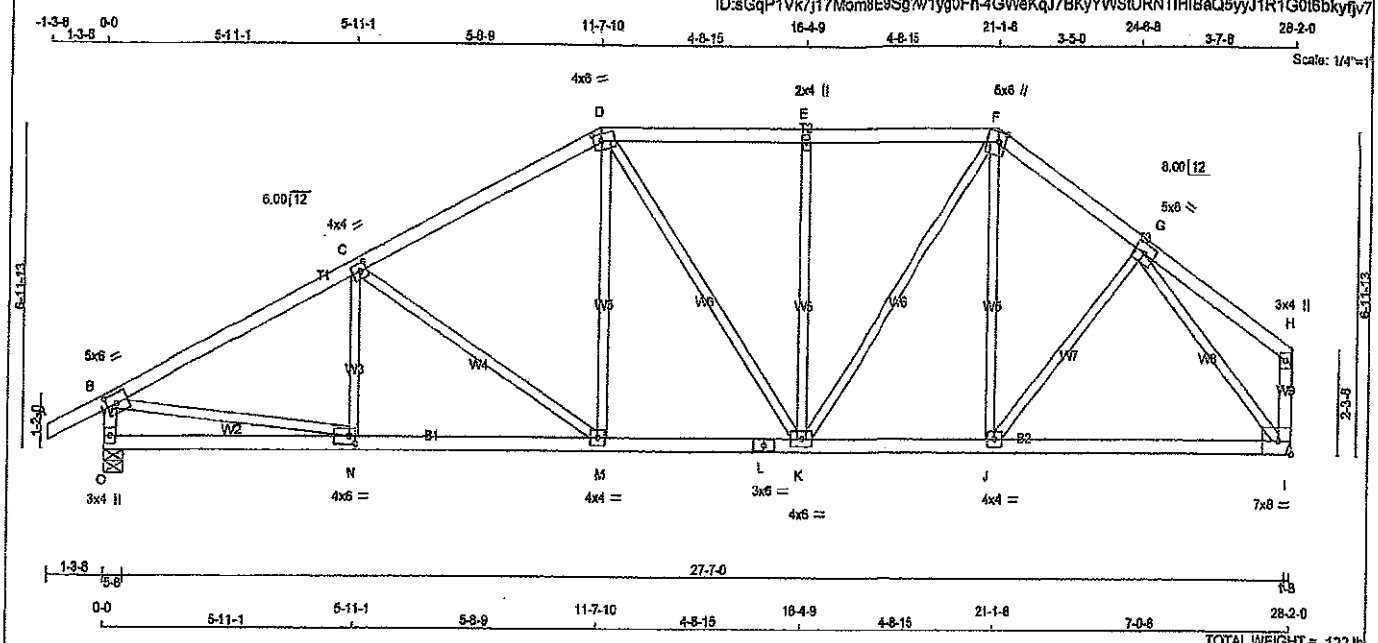

TOTAL WEIGHT = 122 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	5.0	6.0	2.50	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	4.0	6.0	1.75	2.25
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	Edge	1.75
G	TMWW-t	MT20	5.0	6.0		
H	TMV+p	MT20	3.0	4.0		
I	BMW1-t	MT20	7.0	8.0	Edge	
J	BMWW-t	MT20	4.0	4.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMWW-t	MT20	4.0	6.0	2.00	1.75
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
O	1695	0	1695	0	0	5-8	5-8
I	1581	0	1581	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1376	790 / 0	296 / 0	0 / 0	0 / 0	290 / 0	0 / 0
I	1298	721 / 0	296 / 0	0 / 0	0 / 0	282 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.11 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 26	-84.3	-84.3	0.11 (1)	10.00
B-C	-2280 / 0	-84.3	-84.3	0.46 (1)	4.11
C-D	-1836 / 0	-84.3	-84.3	0.41 (1)	4.52
D-E	-1655 / 0	-84.3	-84.3	0.27 (1)	4.89
E-F	-1655 / 0	-84.3	-84.3	0.27 (1)	4.89
F-G	-1547 / 0	-84.3	-84.3	0.15 (1)	5.17
G-H	0 / 16	-84.3	-84.3	0.16 (1)	10.00
O-B	-1628 / 0	0.0	0.0	0.18 (1)	6.48
I-H	-116 / 0	0.0	0.0	0.01 (1)	7.81
O-N	0 / 0	-28.0	-28.0	0.23 (3)	10.00
N-M	0 / 2044	-28.0	-28.0	0.46 (2)	10.00
M-L	0 / 1626	-28.0	-28.0	0.35 (1)	10.00
L-K	0 / 1626	-28.0	-28.0	0.35 (1)	10.00
K-J	0 / 1271	-28.0	-28.0	0.46 (2)	10.00
J-I	0 / 1126	-28.0	-28.0	0.44 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N-C	-116 / 178	0.04 (3)
C-M	-506 / 0	0.47 (1)
M-D	0 / 458	0.10 (2)
D-K	-13 / 50	0.02 (3)
K-E	-486 / 0	0.41 (1)
K-F	0 / 584	0.15 (1)
J-F	0 / 193	0.04 (3)
J-G	0 / 251	0.08 (2)
B-N	0 / 2066	0.46 (1)
G-I	-1783 / 0	0.90 (1)

DWG NO. TAM 45522-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.94")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.94")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.46 (B-C:1) , BC=0.46 (M-N:2) , WB=0.90 (G-I:1) , SSI=0.21 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (G) (INPUT = 0.90)
JSI METAL= 0.81 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285929	T190A	1	1	TRUSS DESC.	

Temarack Roof Truss, Burlington

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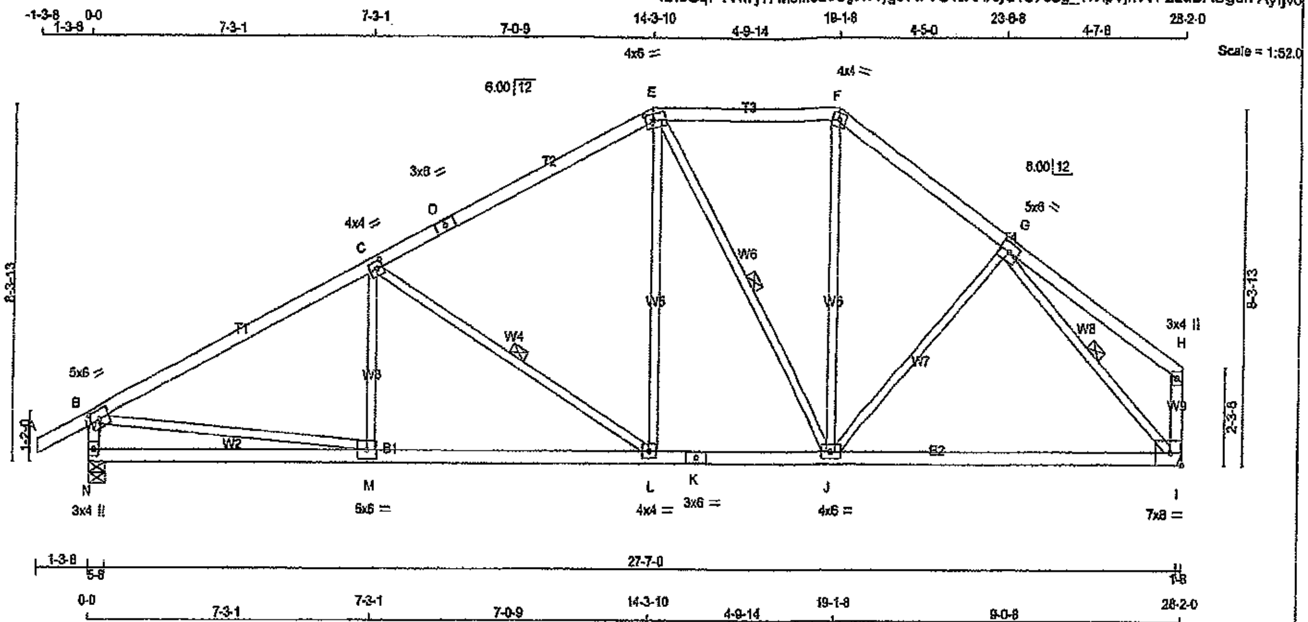

TOTAL WEIGHT = 119 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	4.0	6.0	1.75	2.25
F	TTW-m	MT20	4.0	4.0		
G	TMWW-t	MT20	5.0	8.0		
H	TMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	7.0	8.0		Edge
J	BMWWW-t	MT20	4.0	6.0		
K	BS-t	MT20	3.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMWW-t	MT20	5.0	6.0		
N	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
N	1695	0	1695	0	0	5-8	5-8
I	1581	0	1581	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT.	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1376	790 / 0	296 / 0	0 / 0	0 / 0	290 / 0	0 / 0
I	1298	721 / 0	296 / 0	0 / 0	0 / 0	282 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.75 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-L, E-J, G-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 26	-84.3	-84.3	0.11 (1)	10.00
B- C	-2257 / 0	-84.3	-84.3	0.73 (1)	3.75
C- D	-1585 / 0	-84.3	-84.3	0.63 (1)	4.45
D- E	-1585 / 0	-84.3	-84.3	0.63 (1)	4.45
E- F	-1242 / 0	-84.3	-84.3	0.27 (1)	5.46
F- G	-1512 / 0	-84.3	-84.3	0.24 (1)	5.11
G- H	0 / 24	-84.3	-84.3	0.27 (1)	10.00
N- B	-1614 / 0	0.0	0.0	0.16 (1)	6.51
I- H	-146 / 0	0.0	0.0	0.02 (1)	7.81
N- M	0 / 0	-28.0	-26.0	0.39 (3)	10.00
M- L	0 / 2047	-28.0	-28.0	0.62 (2)	10.00
L- K	0 / 1394	-28.0	-28.0	0.66 (2)	10.00
K- J	0 / 1394	-28.0	-28.0	0.66 (2)	10.00
J- I	0 / 1205	-28.0	-28.0	0.63 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- C	-6 / 300	0.07 (3)
C- L	-779 / 0	0.38 (1)
L- E	0 / 530	0.12 (1)
E- J	-300 / 0	0.18 (1)
J- F	0 / 483	0.11 (2)
J- G	0 / 185	0.04 (3)
B- M	0 / 2062	0.45 (4)
G- I	-1784 / 0	0.54 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.94")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.28")
ALLOWABLE DEFL.(TL)= L/360 (0.94")
CALCULATED VERT. DEFL.(TL) = L/ 717 (0.47")

CSI: TC=0.73 (B-C:1) , BC=0.66 (J-L:2) , WB=0.54 (G-I:1) , SSI=0.26 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1650

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (M) (INPUT = 0.90)
JSI METAL= 0.61 (8) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
9/1/17
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 45523-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 45954	ORWG NO.
285929	T191A	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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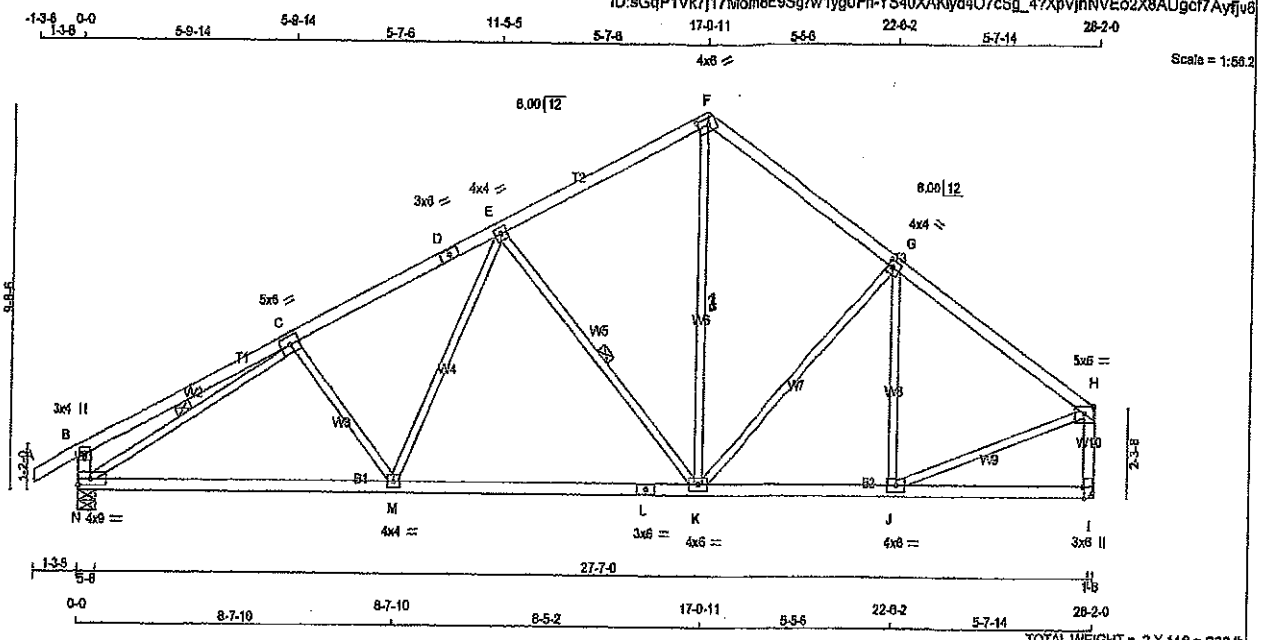


TOTAL WEIGHT = 2 X 118 = 236 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	8.0		
D	TS-t	MT20	3.0	8.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTW-t	MT20	4.0	6.0	2.00	3.50
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW-p	MT20	5.0	8.0	Edge	
I	BMV1+t	MT20	3.0	6.0	Edge	0.50
J	BMWW-t	MT20	4.0	8.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMVW1-t	MT20	4.0	9.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
N	1695	0	1695	0	0	5-8	5-8
I	1581	0	1581	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1376	790 / 0	296 / 0	0 / 0	0 / 0	290 / 0	0 / 0
I	1298	721 / 0	286 / 0	0 / 0	0 / 0	282 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.39 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-K, C-N. F-I

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

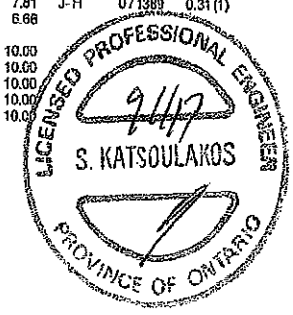
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 26	-84.3	-84.3	0.11 (1)	10.00
B- C	0 / 20	-84.3	-84.3	0.35 (1)	10.00
C- D	-2070 / 0	-84.3	-84.3	0.36 (1)	4.39
D- E	-2070 / 0	-84.3	-84.3	0.36 (1)	4.39
E- F	-1310 / 0	-84.3	-84.3	0.33 (1)	5.27
F- G	-1405 / 0	-84.3	-84.3	0.37 (1)	5.09
G- H	-1548 / 0	-84.3	-84.3	0.36 (1)	4.89
N- B	-311 / 0	0.0	0.0	0.03 (1)	7.81
I- H	-1514 / 0	0.0	0.0	0.19 (1)	6.66
N- M	0 / 1681	-28.0	-28.0	0.74 (2)	10.00
M- L	0 / 1686	-28.0	-28.0	0.71 (2)	10.00
L- K	0 / 1686	-28.0	-28.0	0.71 (2)	10.00
K- J	0 / 1312	-28.0	-28.0	0.41 (2)	10.00
J- I	0 / 0	-28.0	-28.0	0.19 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- M	-208 / 66	0.08 (1)
M- E	0 / 520	0.12 (2)
E- K	-798 / 0	0.40 (1)
K- F	0 / 976	0.22 (1)
K- G	-244 / 0	0.32 (1)
J- G	-293 / 64	0.17 (1)
N- C	-2362 / 0	0.74 (1)
J- H	0 / 1389	0.31 (1)



DWG NO. TAM 45524-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.8	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	48.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.94")
CALCULATED VERT. DEFL.(LL) = L/ 969 (0.22")
ALLOWABLE DEFL.(TL)= L/240 (0.94")
CALCULATED VERT. DEFL.(TL) = L/ 936 (0.36")

CSI: TC=0.39 (G-H:1) , BC=0.74 (M-N:2) , WB=0.74 (C-N:1) , SSI=0.20 (M-N:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (N) (INPUT = 0.90)
JSI METAL= 0.58 (C) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285929	T192	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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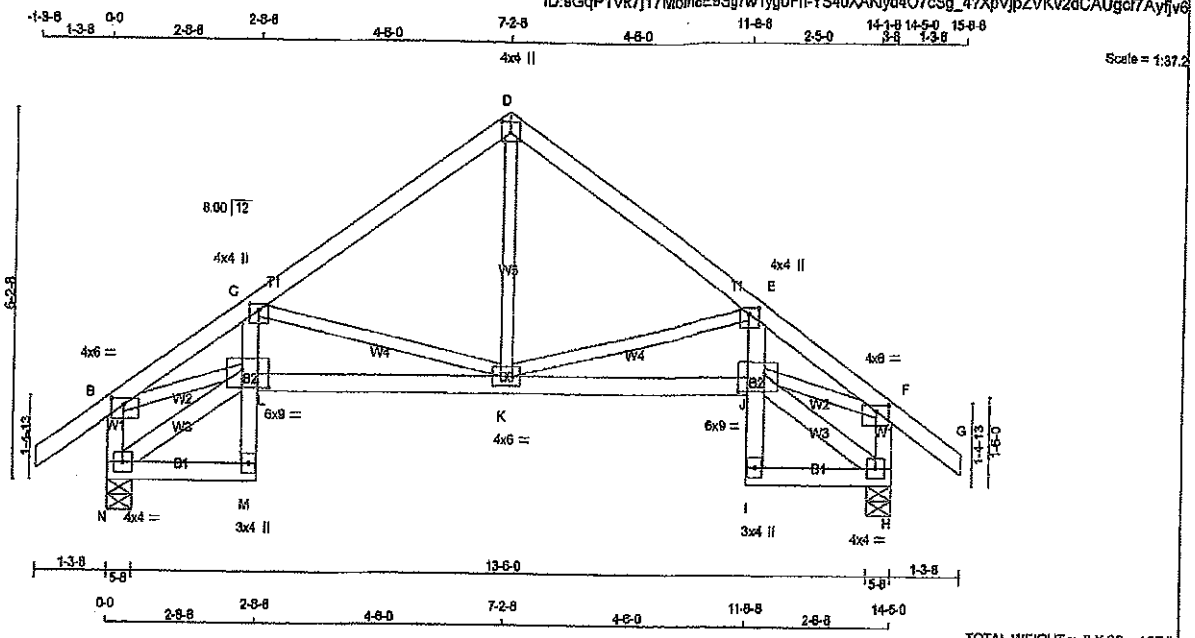


TOTAL WEIGHT = 2 X 69 = 137 lb (M)(F)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
M - C	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
I - E	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	4.0	6.0	1.25	2.75
C	TMWW+p	MT20	4.0	4.0	1.25	2.00
D	TTW+p	MT20	4.0	4.0	2.25	2.00
E	TMWW+p	MT20	4.0	4.0	1.25	2.00
F	TMWW-p	MT20	4.0	6.0	1.25	2.75
H	BMWW1-t	MT20	4.0	4.0		
I	BMV+p	MT20	3.0	4.0		
J	BVMWW-t	MT20	6.0	9.0	3.50	5.75
K	BMWWW-t	MT20	4.0	6.0		
L	BVMWW-t	MT20	6.0	9.0	3.50	5.75
M	BMV+p	MT20	3.0	4.0		
N	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	925	0	925	0	0	5-8	5-8
H	925	0	925	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	743	439 / 0	151 / 0	0 / 0	0 / 0	152 / 0	0 / 0
H	743	439 / 0	151 / 0	0 / 0	0 / 0	152 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

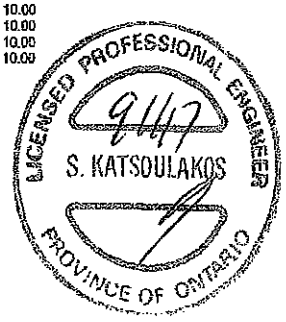
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.90 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	
B- C	-1787 / 0	-84.3	-84.3	0.18 (1)	4.90	
C- D	-834 / 0	-84.3	-84.3	0.24 (1)	6.25	
D- E	-834 / 0	-84.3	-84.3	0.24 (1)	6.25	
E- F	-1787 / 0	-84.3	-84.3	0.16 (1)	4.90	
F- G	0 / 32	-84.3	-84.3	0.11 (1)	10.00	
N- B	-856 / 0	0.0	0.0	0.09 (1)	7.61	
H- F	-856 / 0	0.0	0.0	0.09 (1)	7.61	
N- M	0 / 51	-28.0	-28.0	0.06 (2)	10.00	
M- L	0 / 63	0.0	0.0	0.12 (1)	10.00	
L- C	0 / 410	0.0	0.0	0.18 (1)	10.00	
L- K	0 / 1563	-28.0	-28.0	0.35 (2)	10.00	
K- J	0 / 1563	-28.0	-28.0	0.35 (2)	10.00	
I- J	0 / 63	0.0	0.0	0.12 (1)	10.00	
J- E	0 / 410	0.0	0.0	0.18 (1)	10.00	
I- H	0 / 51	-28.0	-28.0	0.06 (2)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- D	0 / 568	0.13 (1)
K- E	-914 / 0	0.35 (1)
C- K	-914 / 0	0.35 (1)
N- L	-61 / 0	0.01 (1)
J- H	-61 / 0	0.01 (1)
B- L	0 / 1524	0.34 (1)
J- F	0 / 1524	0.34 (1)



DWG NO. TAM 45525-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	45.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.48")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.48")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.24 (C-D:1) , BC=0.35 (K-L:2) , WB=0.35 (C-K:1) , SSI=0.14 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (F) (INPUT = 0.90)
JSI METAL= 0.40 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285931	T193A	2	1	TRUSS DESC.	

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Tamarack Roof Truss, Burlington

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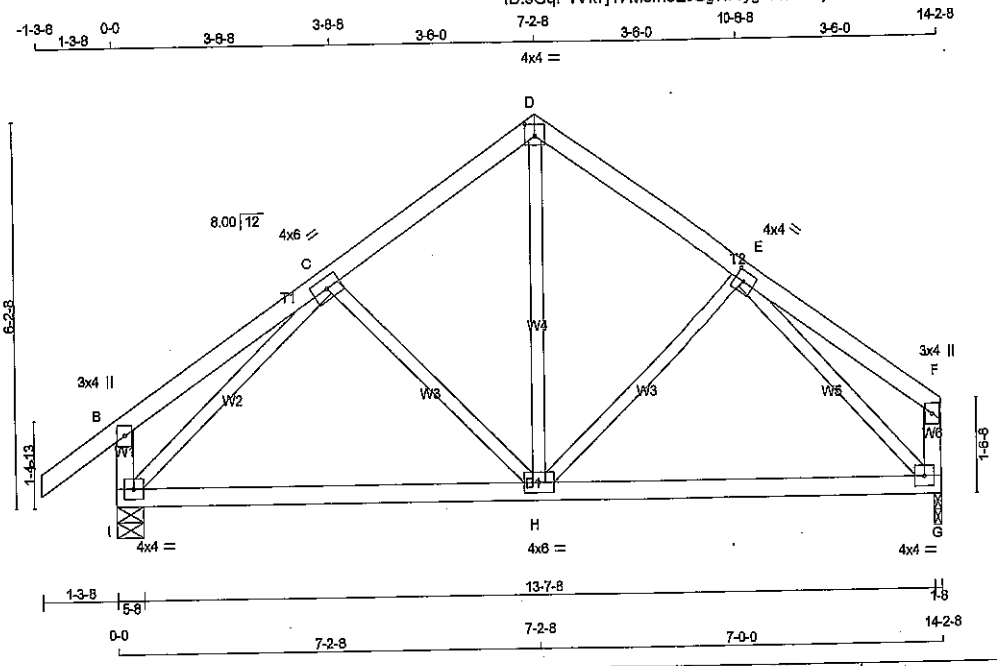

TOTAL WEIGHT = 2 X 60 = 120 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0	2.00	1.75
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BMWWW-t	MT20	4.0	6.0		
I	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	913	0	913	0	0	5-8	5-8
G	797	0	797	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	733	434 / 0	149 / 0	0 / 0	0 / 0	150 / 0	0 / 0
G	655	364 / 0	149 / 0	0 / 0	0 / 0	142 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	
B-C	0 / 19	-84.3	-84.3	0.17 (1)	10.00	
C-D	-532 / 0	-84.3	-84.3	0.13 (1)	6.25	
D-E	-631 / 0	-84.3	-84.3	0.13 (1)	6.25	
E-F	0 / 19	-84.3	-84.3	0.16 (1)	10.00	
I-B	-233 / 0	0.0	0.0	0.02 (1)	7.81	
G-F	-109 / 0	0.0	0.0	0.01 (1)	7.81	
I-H	0 / 623	-28.0	-28.0	0.48 (2)	10.00	
H-G	0 / 599	-28.0	-28.0	0.48 (2)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-H	-158 / 42	0.07 (1)
H-D	0 / 442	0.10 (2)
H-E	-124 / 53	0.05 (1)
I-C	-887 / 0	0.34 (1)
E-G	-859 / 0	0.32 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.47")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.47")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.17/1.00 (B-C:1) , BC=0.48/1.00 (H-I:2) , WB=0.34/1.00 (C-I:1) , SSI=0.17/1.00 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (E) (INPUT = 0.90)
JSI METAL= 0.30 (E) (INPUT = 1.00)

DWG NO. TAM 3348-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285931	T194A	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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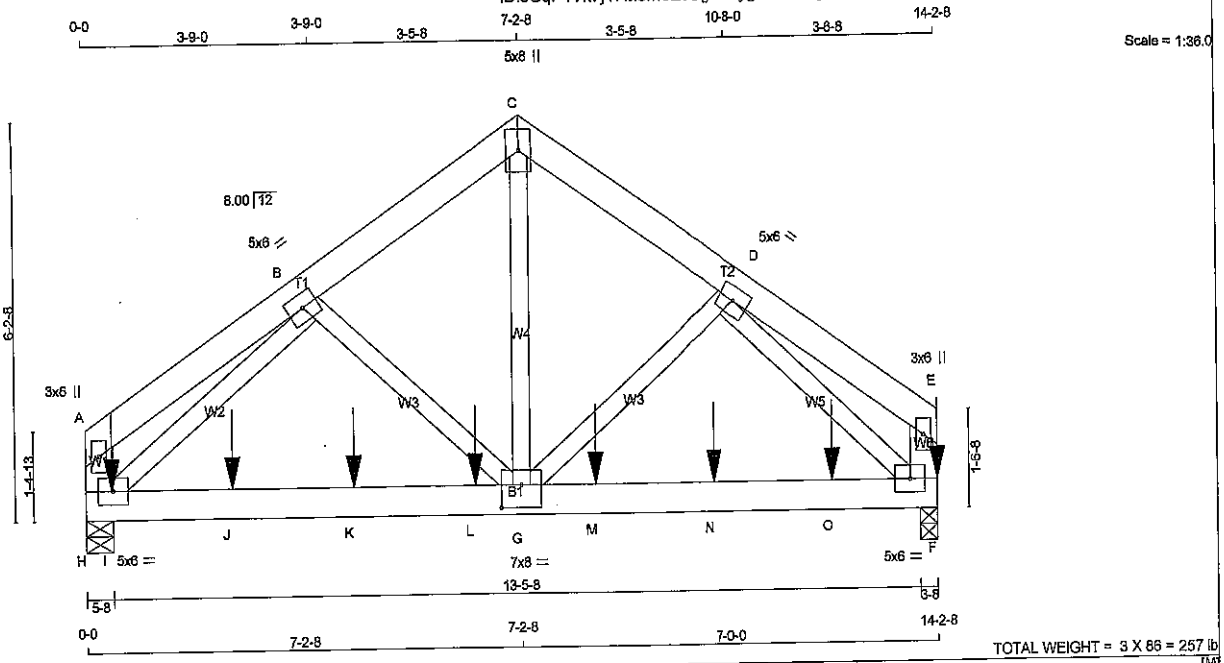

TOTAL WEIGHT = 3 X 86 = 257 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
H - A	2x6	DRY	No.2	SPF
F - E	2x6	DRY	No.2	SPF
H - F	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	TOP
C- E	2	12	TOP
H- A	2	4	SIDE(55.2)
F- E	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
H- F	2	B	SIDE(434.9)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	6.0		
B	TMWW-t	MT20	5.0	6.0		
C	TTW+p	MT20	5.0	8.0		
D	TMWW-t	MT20	5.0	6.0		
E	TMV+p	MT20	3.0	6.0		
F	BMWW1-t	MT20	5.0	6.0		
G	BMWWW-t	MT20	7.0	8.0	4.25	4.00
H	BMWW1-t	MT20	5.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1675.1 lbs FACTORED DOWN AT 5-4, 1667.2 lbs FACTORED DOWN AT 2-5-4, 1667.2 lbs FACTORED DOWN AT 4-5-4, 1667.2 lbs FACTORED DOWN AT 6-5-4, 1552.9 lbs FACTORED DOWN AT 8-5-4, 1552.9 lbs FACTORED DOWN AT 10-5-4, AND 1552.9 lbs FACTORED DOWN AT 12-5-4, AND 1563.1 lbs FACTORED DOWN AT 14-2-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H	7097	0	7097	0	0	5-8	5-8
F	7396	0	7396	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	5735	3333 / 0	1207 / 0	0 / 0	0 / 0	1195 / 0	0 / 0
F	6012	3437 / 0	1303 / 0	0 / 0	0 / 0	1272 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.69 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-20 / 0	-84.3	-84.3	0.05 (1)	6.25	B- G	0 / 742	0.04 (1)
B- C	-5916 / 0	-84.3	-84.3	0.06 (1)	5.69	G- C	0 / 6206	0.33 (1)
C- D	-5915 / 0	-84.3	-84.3	0.06 (1)	5.69	G- D	0 / 931	0.05 (1)
D- E	-22 / 0	-84.3	-84.3	0.04 (1)	6.25	H- B	-5954 / 0	0.49 (1)
H- A	-185 / 0	0.0	0.0	0.00 (1)	7.81	D- F	-5929 / 0	0.46 (1)
F- E	-181 / 0	0.0	0.0	0.00 (1)	7.81			
H- I	0 / 4388	-28.0	-28.0	0.69 (1)	10.00			
I- J	0 / 4388	-28.0	-28.0	0.69 (1)	10.00			
J- K	0 / 4388	-28.0	-28.0	0.69 (1)	10.00			
K- L	0 / 4388	-28.0	-28.0	0.69 (1)	10.00			
L- G	0 / 4388	-28.0	-28.0	0.69 (1)	10.00			
G- M	0 / 4247	-28.0	-28.0	0.69 (1)	10.00			
M- N	0 / 4247	-28.0	-28.0	0.69 (1)	10.00			
N- O	0 / 4247	-28.0	-28.0	0.69 (1)	10.00			
O- F	0 / 4247	-28.0	-28.0	0.69 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
F	14-2-8	-1563	-1563	---	FRONT	VERT	TOTAL
I	5-4	-1675	-1675	---	FRONT	VERT	TOTAL
J	2-5-4	-1667	-1667	---	FRONT	VERT	TOTAL
K	4-5-4	-1667	-1667	---	FRONT	VERT	TOTAL
L	6-5-4	-1667	-1667	---	FRONT	VERT	TOTAL
M	8-5-4	-1553	-1553	---	FRONT	VERT	TOTAL
N	10-5-4	-1553	-1553	---	FRONT	VERT	TOTAL
O	12-5-4	-1553	-1553	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.47")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.47")
CALCULATED VERT. DEFL.(TL) = L/ 776 (0.22")

CSI: TC=0.06/1.00 (C-D:1) , BC=0.69/1.00 (G-H:1) , WB=0.49/1.00 (B-H:1) , SSI=0.80/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (G) (INPUT = 0.90)
JSI METAL= 0.51 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S KATSOULAKOS
PROVINCE OF ONTARIO
1/13/18

DWG NO. TAM 3349-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285930	T195X	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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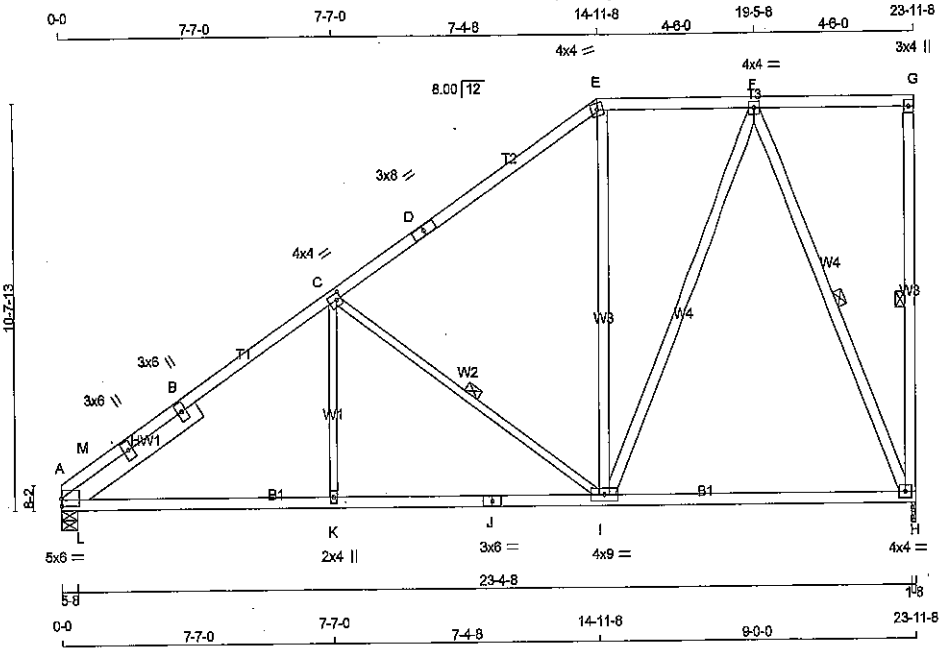


Scale = 1:60.6

TOTAL WEIGHT = 2 X 126 = 251 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
A - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
REINFORCING MEMBERS				
HW1	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
K - C	2x3	DRY	No.2	SPF
C - I	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBMR1-I	MT20	5.0	6.0	2.50	0.25
A	RT+t	MT20	3.0	6.0		
A	RT+t	MT20	3.0	6.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	8.0		
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BS-t	MT20	3.0	6.0		
K	BMW+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	1345	0	1345	0	0	1-8	1-8
A	1345	0	1345	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1104	613 / 0	252 / 0	0 / 0	0 / 0	240 / 0	0 / 0
A	1104	613 / 0	252 / 0	0 / 0	0 / 0	240 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, A

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.68 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-H, C-I, F-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-M	-1722 / 0	-84.3	-84.3	0.12 (1)	4.98
M-B	-1484 / 0	-84.3	-84.3	0.59 (1)	4.68
B-C	-1484 / 0	-84.3	-84.3	0.59 (1)	4.68
C-D	-929 / 0	-84.3	-84.3	0.60 (1)	5.52
D-E	-929 / 0	-84.3	-84.3	0.60 (1)	5.52
E-F	-743 / 0	-84.3	-84.3	0.23 (1)	6.25
F-G	0 / 0	-84.3	-84.3	0.28 (1)	10.00
H-G	-143 / 0	0.0	0.0	0.06 (1)	6.25
A-L	0 / 1127	-28.0	-28.0	0.31 (3)	10.00
L-K	0 / 1400	-28.0	-28.0	0.43 (2)	10.00
K-J	0 / 1400	-28.0	-28.0	0.67 (2)	10.00
J-I	0 / 1400	-28.0	-28.0	0.67 (2)	10.00
I-H	0 / 478	-28.0	-28.0	0.56 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K-C	0 / 342	0.08 (2)
C-I	-810 / 0	0.48 (1)
I-E	0 / 231	0.04 (3)
I-F	0 / 685	0.11 (1)
F-H	-1200 / 0	0.84 (1)
L-M	0 / 457	0.00 (1)
L-B	-370 / 10	0.05 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.60")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.23")
ALLOWABLE DEFL.(TL)= L/360 (0.60")
CALCULATED VERT. DEFL.(TL) = L/ 762 (0.38")

CSI: TC=0.60/1.00 (C-E:1) , BC=0.67/1.00 (I-K:2) , WB=0.84/1.00 (F-H:1) , SSI=9.25/1.00 (C-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

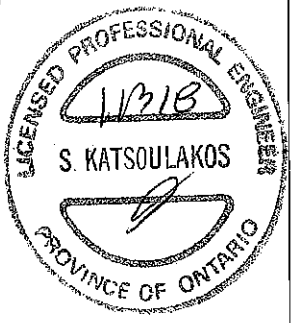
NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (H) (INPUT = 0.90)
JSI METAL= 0.42 (A) (INPUT = 1.00)



DWG NO. TAM 3344-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285930	T196X	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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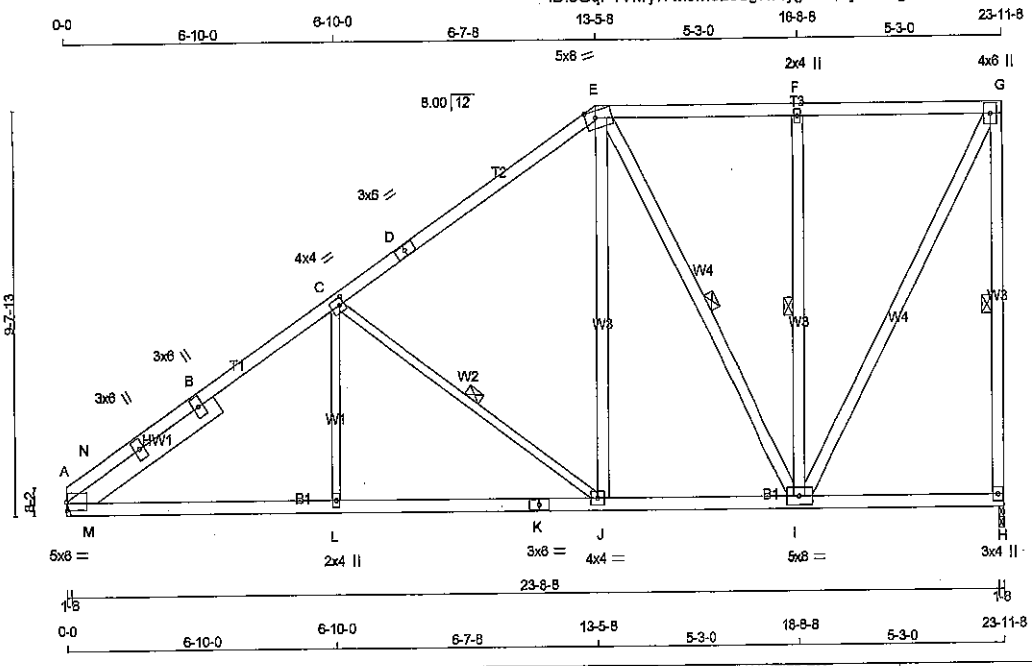


TOTAL WEIGHT = 131 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
A - K	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
REINFORCING MEMBERS				
HW1	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
L - C	2x3	DRY	No.2	SPF
C - J	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBMR1-I	MT20	5.0	6.0	2.50	0.25
A	RT+t	MT20	3.0	6.0		
A	RT+t	MT20	3.0	6.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	5.0	8.0	2.00	3.00
F	TMW+w	MT20	2.0	4.0		
G	TMWW+p	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWWW-t	MT20	5.0	8.0		
J	BMWW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMW+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	1345	0	1345	0	0	1-8	1-8
A	1345	0	1345	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1104	613 / 0	252 / 0	0 / 0	0 / 0	240 / 0	0 / 0
A	1104	613 / 0	252 / 0	0 / 0	0 / 0	240 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.86 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

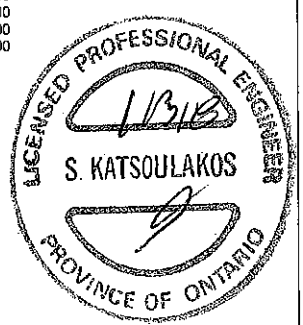
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-H, C-J, E-I, F-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-N	-1718 / 0	-84.3	-84.3	0.10 (1)	5.01	L-C	0 / 343	0.08 (2)
N-B	-1500 / 0	-84.3	-84.3	0.45 (1)	4.86	C-J	-725 / 0	0.34 (1)
B-C	-1500 / 0	-84.3	-84.3	0.45 (1)	4.86	J-E	0 / 589	0.09 (2)
C-D	-1052 / 0	-84.3	-84.3	0.47 (1)	5.51	E-I	-474 / 0	0.28 (1)
D-E	-1052 / 0	-84.3	-84.3	0.47 (1)	5.51	I-F	-548 / 0	0.24 (1)
E-F	-628 / 0	-84.3	-84.3	0.30 (1)	6.25	I-G	0 / 1280	0.21 (1)
F-G	-628 / 0	-84.3	-84.3	0.30 (1)	6.25	M-N	0 / 463	0.00 (1)
H-G	-1284 / 0	0.0	0.0	0.57 (1)	5.89	M-B	-446 / 0	0.06 (1)
A-M	0 / 1108	-28.0	-28.0	0.26 (2)	10.00			
M-L	0 / 1438	-28.0	-28.0	0.43 (2)	10.00			
L-K	0 / 1436	-28.0	-28.0	0.43 (2)	10.00			
K-J	0 / 1436	-28.0	-28.0	0.43 (2)	10.00			
J-I	0 / 858	-28.0	-28.0	0.30 (2)	10.00			
I-H	0 / 0	-28.0	-28.0	0.17 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.80")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.80")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.57/1.00 (G-H:1) , BC=0.43/1.00 (L-M:2) , WB=0.34/1.00 (C-J:1) , SSI=0.22/1.00 (C-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (I) (INPUT = 0.90)
JSI METAL= 0.43 (A) (INPUT = 1.00)

DWG NO. YAM3345-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285930	T197X	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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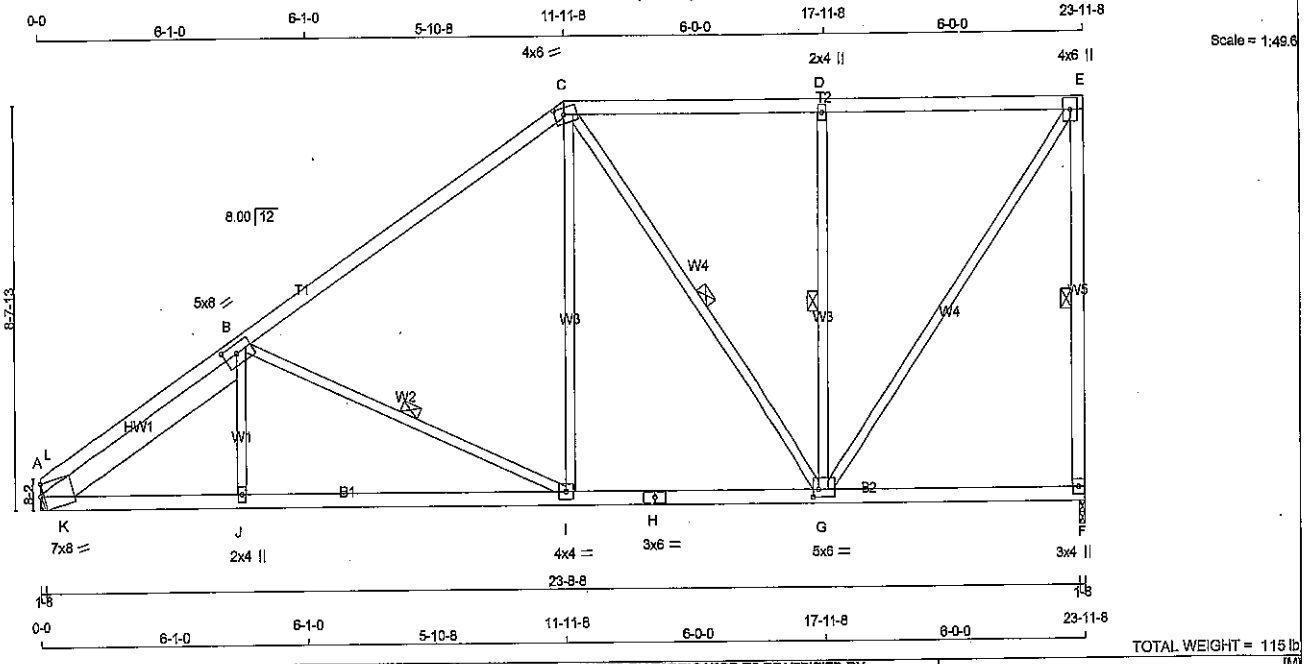

TOTAL WEIGHT = 115 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
A - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
REINFORCING MEMBERS				
HW1	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBMW1-m	MT20	7.0	8.0	3.00	Edge
B	TMWWW-t	MT20	5.0	8.0	2.25	3.50
C	TTWW-m	MT20	4.0	6.0	1.75	2.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW+p	MT20	4.0	8.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	6.0	2.00	1.50
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMW+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
A	1345	0	1345	0	0	IN-SX	IN-SX
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	
F	1345	0	1345	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	1104	613 / 0	252 / 0	0 / 0	0 / 0	240 / 0	0 / 0
F	1104	613 / 0	252 / 0	0 / 0	0 / 0	240 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.83 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-F, B-I, C-G, D-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-L	-1567 / 0	-84.3	-84.3	0.09 (1)		5.19
L-B	-1045 / 0	-84.3	-84.3	0.61 (1)		5.13
B-C	-1203 / 0	-84.3	-84.3	0.75 (1)		4.83
C-D	-781 / 0	-84.3	-84.3	0.54 (1)		5.93
D-E	-781 / 0	-84.3	-84.3	0.54 (1)		5.93
F-E	-1275 / 0	0.0	0.0	0.44 (1)		5.71
A-K	0 / 822	-28.0	-28.0	0.18 (1)		10.00
K-J	0 / 1609	-28.0	-28.0	0.44 (2)		10.00
J-I	0 / 1606	-28.0	-28.0	0.45 (2)		10.00
I-H	0 / 979	-28.0	-28.0	0.36 (2)		10.00
H-G	0 / 979	-28.0	-28.0	0.38 (2)		10.00
G-F	0 / 0	-28.0	-28.0	0.23 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J-B	0 / 332	0.07 (2)
B-I	-698 / 0	0.32 (1)
I-C	0 / 536	0.12 (2)
C-G	-341 / 0	0.26 (1)
G-D	-626 / 0	0.30 (1)
G-E	0 / 1334	0.30 (1)
K-L	0 / 572	0.00 (1)
K-B	-901 / 0	0.22 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.60")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.60")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.75/1.00 (B-C:1) , BC=0.45/1.00 (I-J:2) , WB=0.32/1.00 (B-I:1) , SSI=0.25/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.64 (G) (INPUT = 0.90)
JSI METAL= 0.33 (A) (INPUT = 1.00)

DWG NO. TAM3346-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285929	T198A	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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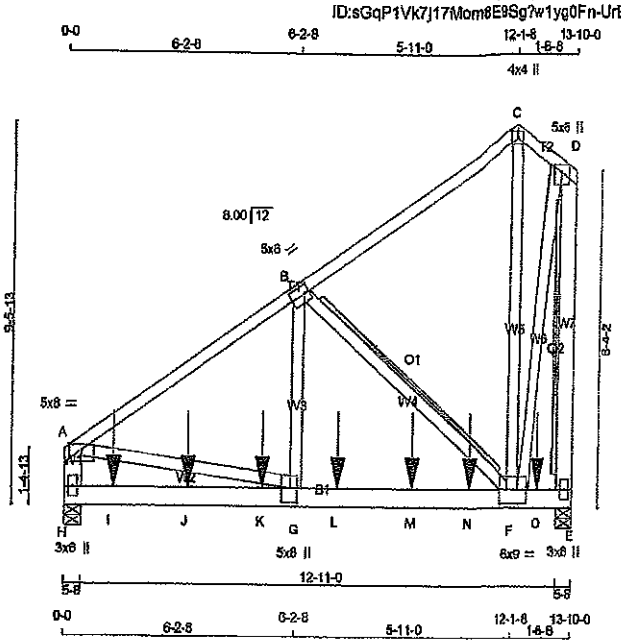

TOTAL WEIGHT = 2 X 99 = 199 lb (M)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
H - A	2x6	DRY	No.2	SPF
E - D	2x6	DRY	No.2	SPF
H - E	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 1 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	TOP
C-O	1	12	TOP
H-A	2	12	TOP
E-O	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
H-E	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	8.0	Edge	
B	TMVW-t	MT20	5.0	6.0	2.50	1.50
C	TTW+p	MT20	4.0	4.0	2.25	2.00
D	TMVW+p	MT20	5.0	6.0	2.25	2.25
E	BMV1+p	MT20	3.0	6.0		
F	BMWWW-t	MT20	8.0	9.0	4.25	2.50
G	BMWW+t	MT20	5.0	8.0	4.25	2.25
H	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H	5563	0	5563	0	0	5-8	5-8
E	5786	0	5786	0	0	5-8	5-6

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	4476	2633 / 0	921 / 0	0 / 0	0 / 0	923 / 0	0 / 0
E	4675	2717 / 0	984 / 0	0 / 0	0 / 0	974 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.78 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x6 DRY SPF No.2 T-BRACE AT B-F, D-E

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	-5435 / 0	-84.3	-84.3	0.58 (1)	3.78	G-B	0 / 4660	0.41 (1)
B-C	-1358 / 0	-84.3	-84.3	0.29 (1)	6.25	B-F	-4582 / 0	0.46 (1)
C-D	-1328 / 0	-84.3	-84.3	0.03 (1)	8.25	F-C	0 / 1209	0.11 (1)
H-A	-3987 / 0	0.0	0.0	0.14 (1)	7.13	A-G	0 / 4908	0.41 (1)
E-D	-5934 / 0	0.0	0.0	0.50 (1)	7.61	F-D	0 / 5244	0.46 (1)
H-I	0 / 0	-28.0	-28.0	0.62 (1)	10.00			
I-J	0 / 0	-28.0	-28.0	0.62 (1)	10.00			
J-K	0 / 0	-28.0	-28.0	0.62 (1)	10.00			
K-G	0 / 0	-28.0	-28.0	0.62 (1)	10.00			
G-L	0 / 4546	-28.0	-28.0	0.73 (1)	10.00			
L-M	0 / 4546	-28.0	-28.0	0.73 (1)	10.00			
M-N	0 / 4546	-28.0	-28.0	0.73 (1)	10.00			
N-F	0 / 4546	-28.0	-28.0	0.73 (1)	10.00			
F-O	0 / 0	-28.0	-28.0	0.25 (1)	10.00			
O-E	0 / 0	-28.0	-28.0	0.25 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
I	1-3-12	-1432	-1432	—	BACK	VERT	TOTAL
J	3-3-12	-1432	-1432	—	BACK	VERT	TOTAL
K	5-3-12	-1432	-1432	—	BACK	VERT	TOTAL
L	7-3-12	-1432	-1432	—	BACK	VERT	TOTAL
M	9-3-12	-1432	-1432	—	BACK	VERT	TOTAL
N	10-10-6	-1317	-1317	—	BACK	VERT	TOTAL
O	12-10-6	-1317	-1317	—	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.46")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.46")
CALCULATED VERT. DEFL.(TL) = L/ 811 (0.20")

CSI: TC=0.58 (A-B:1) , BC=0.73 (F-G:1) , WB=0.46 (B-F:1) , SSI=0.95 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.68 (D) (INPUT = 0.60)
JSI METAL= 0.59 (G) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. T AM 4553-17
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285929	T198A	1	2	TRUSS DESC.	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1432.3 lbs FACTORED DOWN AT 1-3-12, 1432.3 lbs FACTORED DOWN AT 3-3-12, 1432.3 lbs FACTORED DOWN AT 5-3-12, 1432.3 lbs FACTORED DOWN AT 7-3-12, 1432.3 lbs FACTORED DOWN AT 9-3-12, AND 1316.7 lbs FACTORED DOWN AT 10-10-6, AND 1317.1 lbs FACTORED DOWN AT 12-10-6 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
9/14/17
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM45534-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285929	G199	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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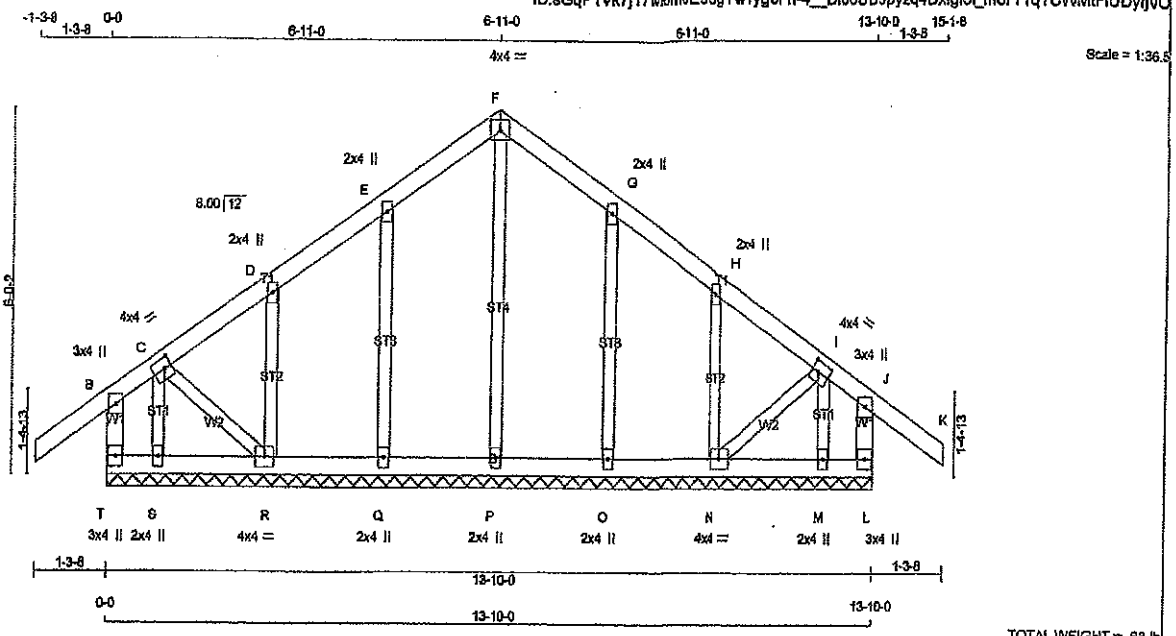

TOTAL WEIGHT = 63 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
T - B	2x4	DRY	No.2	SPF
A - F	2x4	DRY	No.2	SPF
F - K	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D, E, G, H						
D	TMV+w	MT20	2.0	4.0		
F	TTW-p	MT20	4.0	4.0	2.25	2.00
I	TMWW-t	MT20	4.0	4.0	2.00	1.50
J	TMV+p	MT20	3.0	4.0		
L	BMV1+p	MT20	3.0	4.0		
M, O, P, Q, S						
M	BMW1+w	MT20	2.0	4.0		
N	BMWW1-t	MT20	4.0	4.0		
R	BMWW1-t	MT20	4.0	4.0		
T	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH
T-B	-231 / 0	0.0	0.0	0.02 (1)	7.81	
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	
B-C	-43 / 0	-84.3	-84.3	0.11 (1)	6.25	
C-D	-22 / 0	-84.3	-84.3	0.04 (1)	6.25	
D-E	-20 / 0	-84.3	-84.3	0.05 (1)	6.25	
E-F	-27 / 0	-84.3	-84.3	0.05 (1)	6.25	
F-G	-27 / 0	-84.3	-84.3	0.05 (1)	6.25	
G-H	-20 / 0	-84.3	-84.3	0.05 (1)	6.25	
H-I	-22 / 0	-84.3	-84.3	0.04 (1)	6.25	
I-J	-43 / 0	-84.3	-84.3	0.11 (1)	6.25	
J-K	0 / 32	-84.3	-84.3	0.11 (1)	10.00	
L-J	-231 / 0	0.0	0.0	0.02 (1)	7.81	
T-S	0 / 0	-28.0	-28.0	0.02 (2)	10.00	
S-R	0 / 0	-28.0	-28.0	0.02 (2)	10.00	
R-O	0 / 16	-28.0	-28.0	0.02 (2)	10.00	
Q-P	0 / 15	-28.0	-28.0	0.02 (2)	10.00	
P-O	0 / 15	-28.0	-28.0	0.02 (2)	10.00	
O-N	0 / 16	-28.0	-28.0	0.02 (2)	10.00	
N-M	0 / 0	-28.0	-28.0	0.02 (2)	10.00	
M-L	0 / 0	-28.0	-28.0	0.02 (2)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P-F	-122 / 0	0.07 (1)
Q-E	-190 / 0	0.06 (1)
R-D	-169 / 0	0.03 (1)
S-C	-57 / 0	0.01 (1)
O-G	-190 / 0	0.06 (1)
N-H	-169 / 0	0.03 (1)
M-I	-57 / 0	0.01 (1)
C-R	0 / 30	0.01 (1)
N-I	0 / 30	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11 (A-B:1) , BC=0.02 (N-O:2) , WB=0.07 (F-P:1) , SSI=0.08 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (E) (INPUT = 0.90)
JSI METAL= 0.05 (G) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
9/11/17
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 45540 17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285929	T200	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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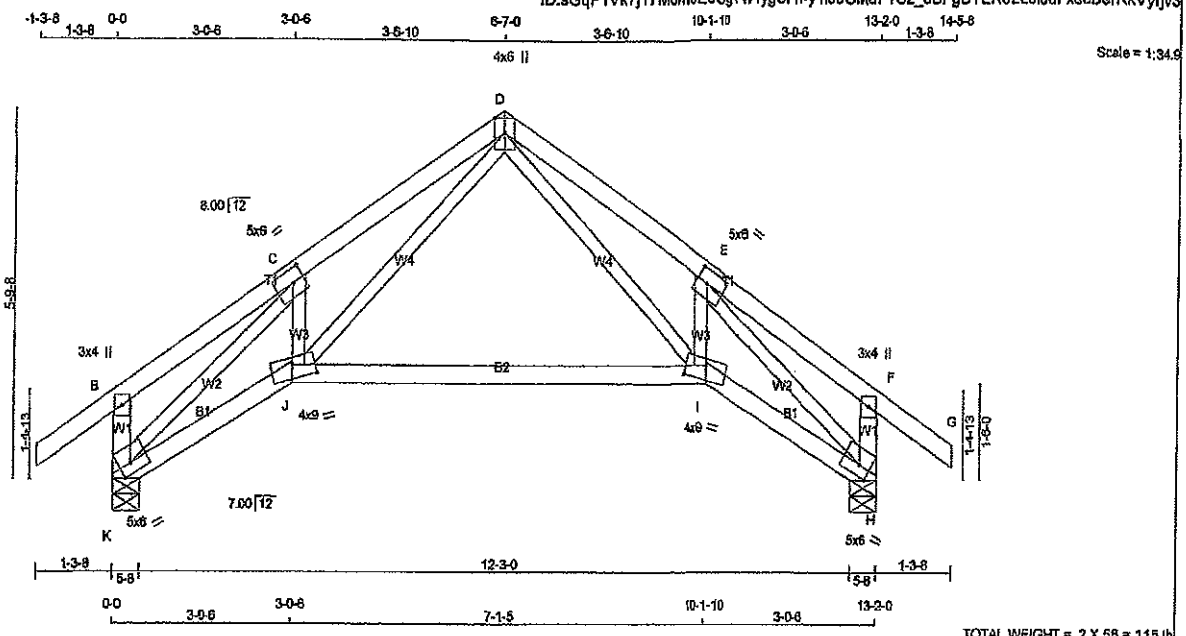


TOTAL WEIGHT = 2 X 58 = 115 lb (M)(F)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.25
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	5.0	6.0	2.50	2.25
F	TMV+p	MT20	3.0	4.0		
H	BMWW1-t	MT20	5.0	6.0	Edge	2.25
I	BBWW-m	MT20	4.0	9.0	2.75	4.50
J	BBWW-m	MT20	4.0	9.0	2.75	4.50
K	BMWW1-t	MT20	5.0	6.0	Edge	2.25

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	655	0	655	0	0	5-8	5-8
H	655	0	655	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	685	407 / 0	138 / 0	0 / 0	0 / 0	140 / 0	0 / 0
H	685	407 / 0	138 / 0	0 / 0	0 / 0	140 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.43 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	D-I	0 / 615	0.18 (1)
B-C	-4 / 10	-84.3	-84.3	0.10 (1)	10.00	I-E	0 / 235	0.05 (2)
C-D	-1346 / 0	-84.3	-84.3	0.16 (1)	5.43	J-D	0 / 615	0.18 (1)
D-E	-1346 / 0	-84.3	-84.3	0.16 (1)	5.43	J-C	0 / 235	0.05 (2)
E-F	-4 / 10	-84.3	-84.3	0.10 (1)	10.00	K-C	-1658 / 0	0.50 (1)
F-G	0 / 32	-84.3	-84.3	0.11 (1)	10.00	E-H	-1658 / 0	0.50 (1)
K-B	-224 / 0	0.0	0.0	0.02 (1)	7.81			
H-F	-224 / 0	0.0	0.0	0.02 (1)	7.81			
K-J	0 / 1282	-28.0	-28.0	0.25 (1)	10.00			
J-I	0 / 570	-28.0	-28.0	0.48 (2)	10.00			
I-H	0 / 1282	-28.0	-28.0	0.25 (1)	10.00			

LICENSED PROFESSIONAL ENGINEER
9/11/17
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM45532-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.44")
CALCULATED VERT. DEFL.(LL) = L/ 762 (0.21")
ALLOWABLE DEFL.(TL)= L/360 (0.44")
CALCULATED VERT. DEFL.(TL) = L/ 461 (0.34")

CSI: TC=0.16 (D-E:1) , BC=0.48 (I-J:2) , WB=0.50 (C-K:1) , SSI=0.14 (I-J:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

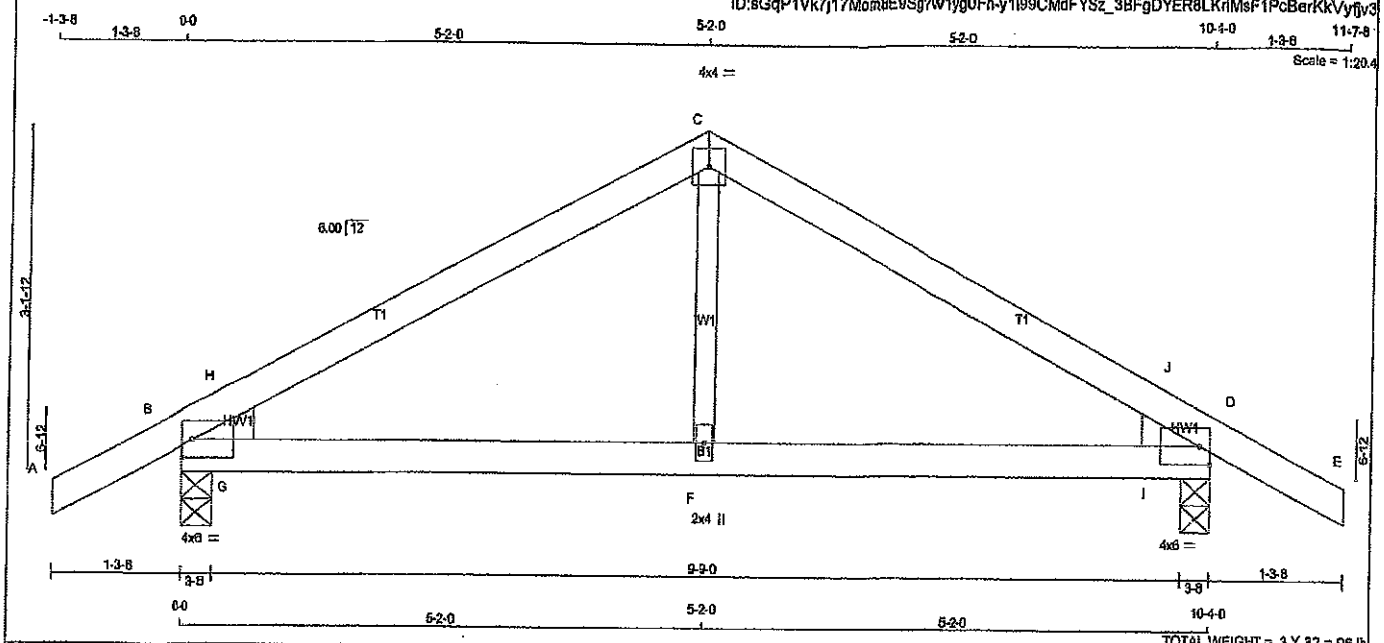
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (E) (INPUT = 0.90)
JSI METAL= 0.41 (H) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43854	DRWG NO.
285929	T201	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington



TOTAL WEIGHT = 3 X 32 = 96 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-I	MT20	4.0	6.0	2.00	1.25
C	TTW-p	MT20	4.0	4.0		
D	TMBH1-I	MT20	4.0	6.0		Edge
F	BMW+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG	HEEL
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	WEDGE
B	694	0	694	0	0	3-8	3-8	2x4 L
D	694	0	694	0	0	3-8	3-8	2x4 R

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	554	334 / 0	109 / 0	0 / 0	0 / 0	111 / 0	0 / 0
D	554	334 / 0	109 / 0	0 / 0	0 / 0	111 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 17	-84.3	-84.3	0.11 (1)	10.00	
B-H	-664 / 0	-84.3	-84.3	0.12 (1)	6.25	
H-C	-653 / 0	-84.3	-84.3	0.24 (1)	6.25	
C-J	-653 / 0	-84.3	-84.3	0.24 (1)	6.25	
J-D	-664 / 0	-84.3	-84.3	0.12 (1)	6.25	
D-E	0 / 17	-84.3	-84.3	0.11 (1)	10.00	
B-G	0 / 575	-28.0	-28.0	0.23 (1)	10.00	
G-F	0 / 575	-28.0	-28.0	0.27 (1)	10.00	
F-I	0 / 575	-28.0	-28.0	0.27 (1)	10.00	
I-D	0 / 575	-28.0	-28.0	0.22 (1)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F-C	0 / 291	0.07 (2)
G-H	-166 / 161	0.00 (1)
I-J	-165 / 162	0.00 (1)

DWG NO. TAM 45533-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(65 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.24 (C-H:1) , BC=0.27 (F-G:1) , WB=0.07 (C-F:2) , SSI=0.15 (E-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.68 (D) (INPUT = 0.90)
JSI METAL= 0.17 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285929	G201	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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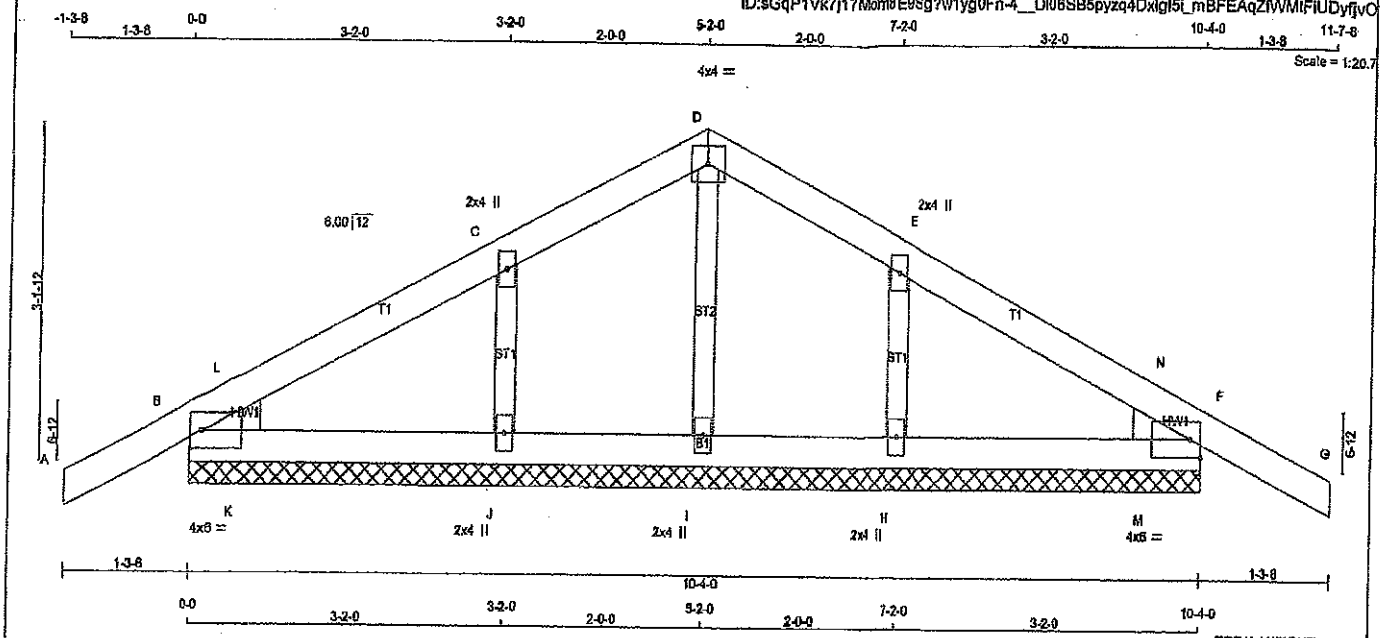

TOTAL WEIGHT = 35 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-I	MT20	4.0	6.0	2.00	1.25
C	TMW+w	MT20	2.0	4.0		
D	TTW-p	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TMBH1-I	MT20	4.0	6.0		Edge
H, I, J						
H	BMW1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

HEEL
WEDGE
2x4 L
2x4 R
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 17	-84.3	-84.3	0.11 (1)	10.00
B- L	-152 / 0	-84.3	-84.3	0.03 (3)	6.25
L- C	-107 / 0	-84.3	-84.3	0.08 (1)	6.25
C- D	-121 / 0	-84.3	-84.3	0.08 (1)	6.25
D- E	-121 / 0	-84.3	-84.3	0.08 (1)	6.25
E- N	-107 / 0	-84.3	-84.3	0.08 (1)	6.25
N- F	-152 / 0	-84.3	-84.3	0.03 (3)	6.25
F- G	0 / 17	-84.3	-84.3	0.11 (1)	10.00
B- K	0 / 105	-28.0	-28.0	0.06 (1)	10.00
K- J	0 / 105	-28.0	-28.0	0.07 (2)	10.00
J- I	0 / 97	-28.0	-28.0	0.07 (2)	10.00
I- H	0 / 97	-28.0	-28.0	0.07 (2)	10.00
H- M	0 / 105	-28.0	-28.0	0.07 (2)	10.00
M- F	0 / 105	-28.0	-28.0	0.06 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- D	-18 / 42	0.01 (3)
J- C	-246 / 0	0.04 (1)
H- E	-246 / 0	0.04 (1)
K- L	-50 / 59	0.00 (1)
M- N	-49 / 59	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11 (A-B:1) , BC=0.07 (J-K:2) , WB=0.04 (C-J:1) , BSI=0.09 (C-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	822	2284	1658

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.32 (F) (INPUT = 0.90)
JSI METAL= 0.05 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
9/11
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 4554/-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285929	T202TCX	9	1	TRUSS DESC.	

Tamerack Roof Truss, Burlington

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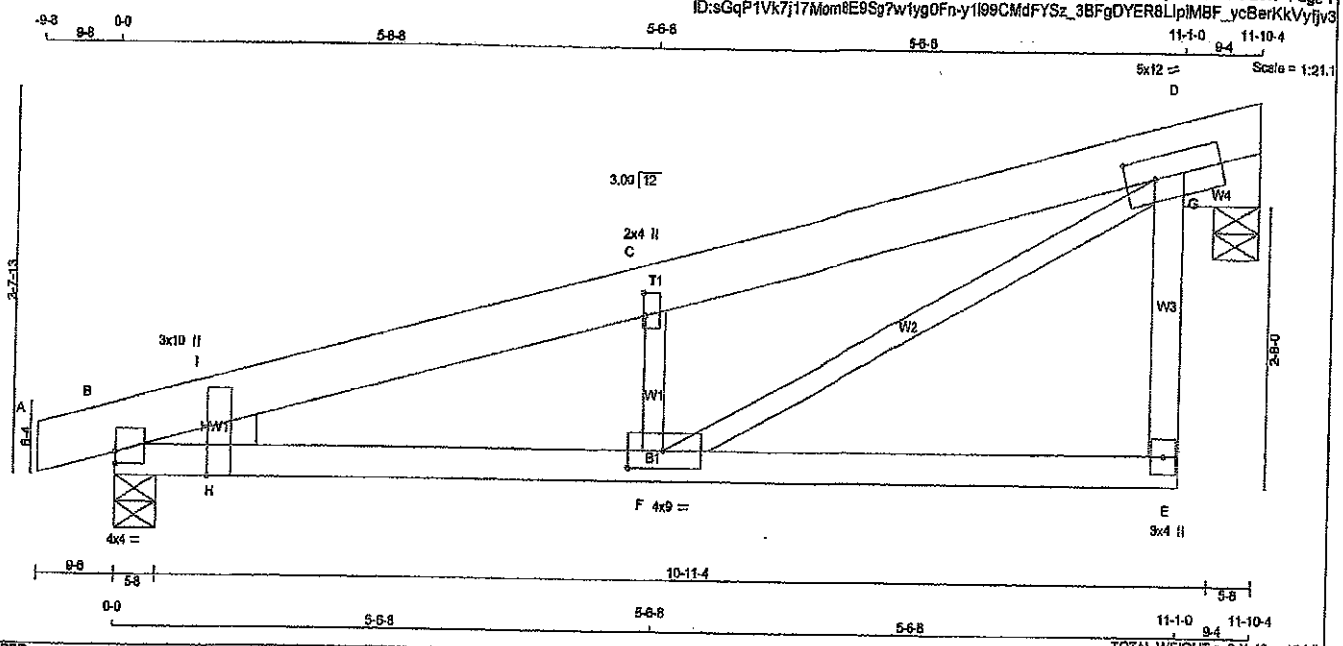

TOTAL WEIGHT = 9 X 48 = 431 lb (M)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-t	MT20	4.0	4.0	1.25	
B	WP+t	MT20	3.0	10.0	Edge	11.50
C	TMW+w	MT20	2.0	4.0	2.50	0.25
O	TMVWW1-t	MT20	5.0	12.0	2.50	3.75
E	BMV+p	MT20	3.0	4.0		
F	BMWW-t	MT20	4.0	9.0	2.00	4.25

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	RECRD BRG IN-SX	HEEL WEDGE
J(D)	593	0	593	0	0	5-8	5-8	
B	722	0	722	0	0	5-8	5-8	2x4 L

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
J(D)	487	270 / 0	111 / 0	0 / 0	0 / 0	106 / 0	0 / 0
B	583	340 / 0	122 / 0	0 / 0	0 / 0	121 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J(D), B

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A-B	-4 / 0	-84.3	-84.3	0.02 (1)	10.00
B-I	-1371 / 0	-84.3	-84.3	0.04 (1)	6.25
I-C	-1265 / 0	-84.3	-84.3	0.10 (1)	6.25
C-O	-1259 / 0	-84.3	-84.3	0.19 (1)	6.25
E-G	0 / 113	0.0	0.0	0.37 (1)	10.00
G-D	0 / 113	0.0	0.0	0.37 (1)	10.00
B-H	0 / 1231	-28.0	-28.0	0.22 (1)	10.00
H-F	0 / 1231	-28.0	-28.0	0.32 (2)	10.00
F-E	0 / 104	-28.0	-28.0	0.19 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F-C	-460 / 0	0.07 (1)
F-D	0 / 1271	0.29 (1)
H-I	0 / 145	0.00 (1)
D-J	-628 / 0	0.03 (1)
G-J	0 / 578	0.09 (1)

LICENSED PROFESSIONAL ENGINEER
9/11/17
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 45534-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.37")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.37")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.37 (E-G:1) , BC=0.32 (F-H:2) , WB=0.29 (D-F:1) , SSI=0.16 (D-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	822	2284	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (F) (INPUT = 0.90)
JSI METAL= 0.44 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285929	G202	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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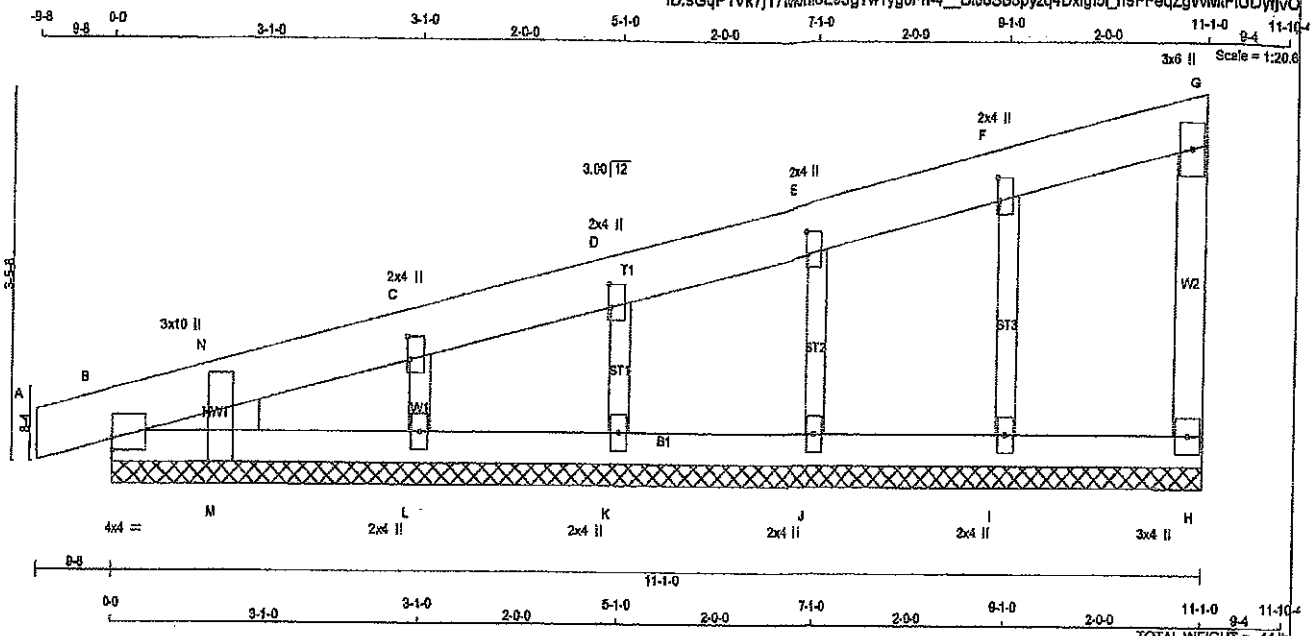

TOTAL WEIGHT = 44 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - G	2x6	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
B - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	4.0	4.0	1.25	
B	WP+I	MT20	3.0	10.0	Edge	11.50
C, D, E, F						
C	TMW+w	MT20	2.0	4.0	2.50	0.25
G	TMV+p	MT20	3.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I, J, K, L						
I	BMW1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

HEEL
WEDGE
2x4 L
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A-B	-4 / 0	-84.3	-84.3	0.02 (1)		10.00
B-N	-57 / 0	-84.3	-84.3	0.04 (1)		6.25
N-C	-11 / 4	-84.3	-84.3	0.05 (1)		6.25
C-D	-20 / 0	-84.3	-84.3	0.05 (1)		8.25
D-E	-8 / 0	-84.3	-84.3	0.02 (1)		10.00
E-F	-4 / 0	-84.3	-84.3	0.02 (1)		10.00
F-G	-4 / 0	-84.3	-84.3	0.02 (1)		10.00
H-G	-72 / 0	0.0	0.0	0.01 (1)		7.81
B-M	0 / 27	-28.0	-28.0	0.03 (1)		10.00
M-L	0 / 27	-28.0	-28.0	0.04 (2)		10.00
L-K	0 / 14	-28.0	-28.0	0.04 (2)		10.00
K-J	0 / 9	-28.0	-28.0	0.02 (2)		10.00
J-I	0 / 4	-28.0	-28.0	0.03 (2)		10.00
I-H	0 / 0	-28.0	-28.0	0.09 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I-F	-165 / 0	0.03 (1)
J-E	-171 / 0	0.03 (1)
K-D	-138 / 0	0.02 (1)
L-C	-256 / 0	0.04 (1)
M-N	0 / 63	0.09 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.05 (C-D:1) , BC=0.04 (L-M:2) , WB=0.04 (C-L:1) , SSI=0.09 (C-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (C) (INPUT = 0.90)
JSI METAL= 0.05 (L) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
9/11/17
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 45542-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285930	T1380X	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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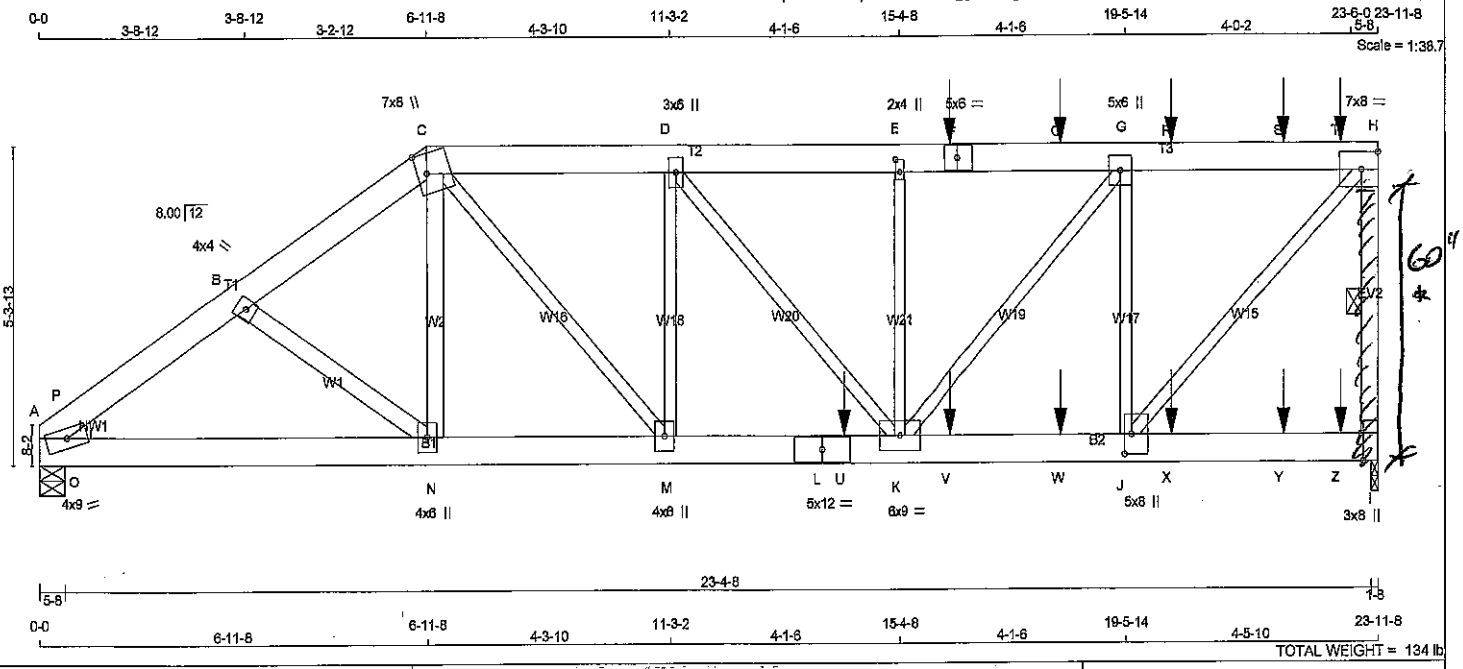

TOTAL WEIGHT = 134 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
A - L	2x6	DRY	2100F 1.8E	SPF
L - I	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
B - N	2x4	DRY	No.2	SPF
N - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBMH1-m	MT20	4.0	9.0		
B	TMW+w	MT20	4.0	4.0		
C	TTWW+m	MT20	7.0	8.0	4.00	2.00
D	TMWW+t	MT20	3.0	6.0		
E	TMW+w	MT20	2.0	4.0	2.50	1.00
F	TS-t	MT20	5.0	6.0		
G	TMWW+t	MT20	5.0	6.0		
H	TMW-t	MT20	7.0	8.0		Edge
I	BMV1+p	MT20	3.0	6.0	Edge	0.50
J	BMWW+t	MT20	5.0	8.0	4.00	1.50
K	BMWWW-t	MT20	6.0	9.0		
L	BS-t	MT20	5.0	12.0		
M	BMWW+t	MT20	4.0	6.0		
N	BMWW+t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 101.4 lbs FACTORED DOWN AT 16-3-12, 101.4 lbs FACTORED DOWN AT 18-3-12, 101.4 lbs FACTORED DOWN AT 20-3-12, AND 101.4 lbs FACTORED DOWN AT 22-3-12, AND 116.4 lbs FACTORED DOWN AT 23-3-12 ON TOP CHORD, AND 1212.0 lbs FACTORED DOWN AT 14-4-8, 71.7 lbs FACTORED DOWN AT 16-3-12, 71.7 lbs FACTORED DOWN AT 18-3-12, 71.7 lbs FACTORED DOWN AT 20-3-12, AND 71.7 lbs FACTORED DOWN AT 22-3-12, AND 80.4 lbs FACTORED DOWN AT 23-3-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

* 60" VERTICAL SCAB

SCAB JOINT # I & L WITH 2 X 4 #2 SPF 4" LONG 1 SIDE(S) USING 1 ROW(S) 3 1/2" ANDY'S SPECIAL NAILS AT 3" o/c STAGGERED (16 NAILS/SCAB) REQUIRED FOR 1 1/2" BRG PROVISION.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
A	1944	0	1944	0	0	5-8	5-8	2x4 L
I	2690	0	2690	0	0	1-8	1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	1594	888 / 0	361 / 0	0 / 0	0 / 0	345 / 0	0 / 0
I	2214	1220 / 0	511 / 0	0 / 0	0 / 0	484 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.46 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF H-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI	MAX (LC)	MAX. UNBRAC LENGTH
A- P	-3129 / 0	-84.3	-84.3	0.14 (1)		4.64
P- B	-2905 / 0	-84.3	-84.3	0.13 (1)		4.79
B- C	-2689 / 0	-84.3	-84.3	0.13 (1)		4.94
C- D	-3106 / 0	-84.3	-84.3	0.17 (1)		4.64
D- E	-3232 / 0	-84.3	-84.3	0.15 (1)		4.58
E- F	-3232 / 0	-84.3	-84.3	0.26 (1)		4.46
F- Q	-3232 / 0	-84.3	-84.3	0.26 (1)		4.46
Q- G	-3232 / 0	-84.3	-84.3	0.26 (1)		4.46
G- R	-2061 / 0	-84.3	-84.3	0.24 (1)		5.35
R- S	-2061 / 0	-84.3	-84.3	0.24 (1)		5.35
S- T	-2061 / 0	-84.3	-84.3	0.24 (1)		5.35
T- H	-2061 / 0	-84.3	-84.3	0.24 (1)		5.35
I- H	-2555 / 0	0.0	0.0	0.36 (1)		4.25
A- O	0 / 2395	-28.0	-28.0	0.16 (1)		10.00
O- N	0 / 2395	-28.0	-28.0	0.16 (1)		10.00
N- M	0 / 2239	-28.0	-28.0	0.20 (1)		10.00
M- L	0 / 3106	-28.0	-28.0	0.45 (1)		10.00
L- U	0 / 3106	-28.0	-28.0	0.45 (1)		10.00
U- K	0 / 3106	-28.0	-28.0	0.45 (1)		10.00
K- V	0 / 2061	-28.0	-28.0	0.21 (1)		10.00
V- W	0 / 2061	-28.0	-28.0	0.21 (1)		10.00
W- J	0 / 2061	-28.0	-28.0	0.21 (1)		10.00
J- X	0 / 0	-28.0	-28.0	0.08 (2)		10.00
X- Y	0 / 0	-28.0	-28.0	0.08 (2)		10.00
Y- Z	0 / 0	-28.0	-28.0	0.08 (2)		10.00
Z- I	0 / 0	-28.0	-28.0	0.08 (2)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- N	-202 / 0	0.05 (1)
N- C	0 / 293	0.05 (2)
J- H	0 / 3045	0.75 (1)
C- M	0 / 1304	0.32 (1)
J- G	-2089 / 0	0.79 (1)
M- D	-530 / 0	0.20 (1)
K- G	0 / 1812	0.45 (1)
D- K	0 / 195	0.05 (1)
K- E	-382 / 0	0.14 (1)
O- P	0 / 331	0.00 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
F	16-3-12	-101	-101	---	FRONT	VERT	TOTAL
Q	18-3-12	-101	-101	---	FRONT	VERT	TOTAL
R	20-3-12	-101	-101	---	FRONT	VERT	TOTAL
S	22-3-12	-101	-101	---	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER S. KATSOULAKOS PROVINCE OF ONTARIO

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.80")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.80")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.38/1.00 (H-I:1) , BC=0.45/1.00 (K-M:1) , WB=0.79/1.00 (G-J:1) , SSI=0.65/1.00 (K-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (A) (INPUT = 0.90)
JSI METAL= 0.97 (L) (INPUT = 1.00)

DWG NO. TAM 4956 -18
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
T	23-3-12	-116	-116	---	FRONT	VERT	TOTAL
U	14-4-8	-1212	-1212	---	FRONT	VERT	TOTAL
V	16-3-12	-41	-72	---	FRONT	VERT	TOTAL
W	18-3-12	-41	-72	---	FRONT	VERT	TOTAL
X	20-3-12	-41	-72	---	FRONT	VERT	TOTAL
Y	22-3-12	-41	-72	---	FRONT	VERT	TOTAL
Z	23-3-12	-46	-80	---	FRONT	VERT	TOTAL

DWG NO. TAM4058 -R2

STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285929	P1	2	1	TRUSS DESC.	

Tamareck Roof Truss, Burlington

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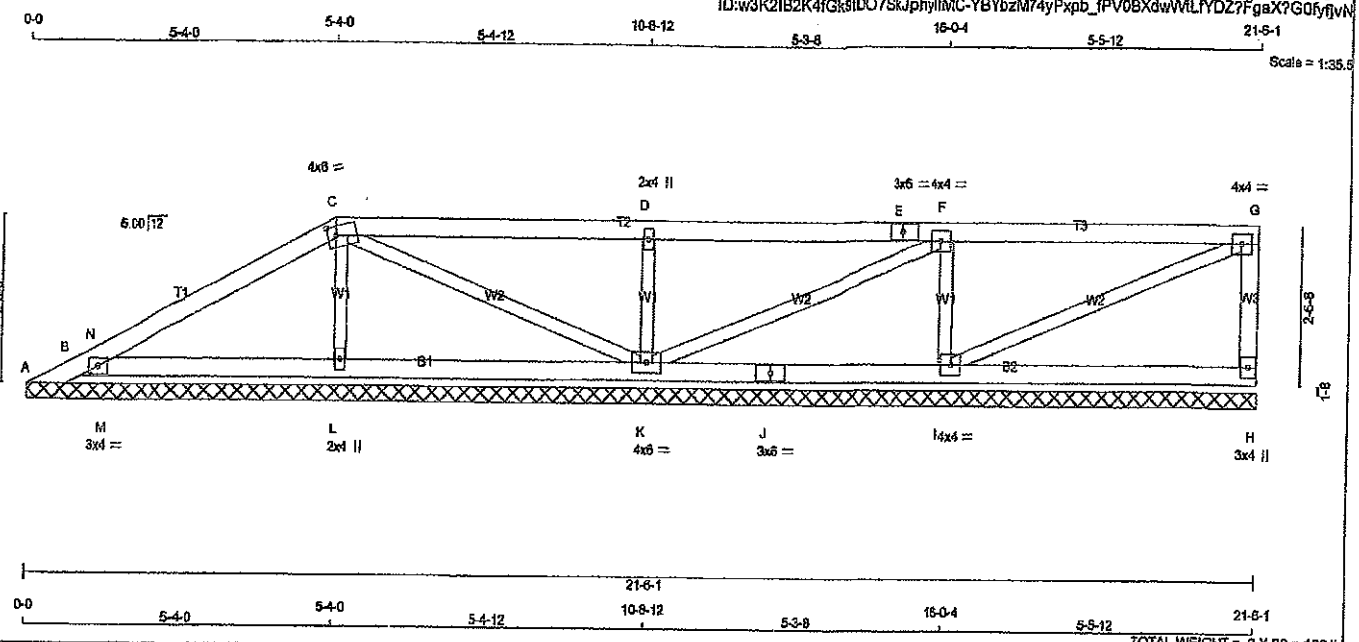


TOTAL WEIGHT = 2 X 69 = 138 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
B - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

BEARING NOTE: GAP BETWEEN INSIDE OF TOP CHORD BEARING AND FIRST DIAGONAL OR VERTICAL WEB SHALL NOT EXCEED 0.5 INCHES.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW-m	MT20	4.0	6.0	1.75	1.75
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMW-t	MT20	4.0	4.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWW1-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW1-t	MT20	4.0	6.0		
L	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	RECRD BRG IN-SX
A	-129	0	0	0	-129	21-6-1 (11-11[3/8]	3-8-1
H	250	0	250	0	0	21-6-1 (11-11[3/8]	3-8-1
B	506	0	506	0	0	21-6-1 (11-11[3/8]	3-8-1
L	446	0	446	0	0	21-6-1 (11-11[3/8]	3-8-1
I	656	0	656	0	0	21-6-1 (11-11[3/8]	3-8-1
K	667	0	667	0	0	21-6-1 (11-11[3/8]	3-8-1

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): H, L, I, K

PROVIDE ANCHORAGE AT BEARING JOINT A FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS
1ST LCASE — MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	-104	0 / -64	0 / -23	0 / 0	0 / 0	0 / -17	0 / 0
H	205	115 / 0	46 / 0	0 / 0	0 / 0	44 / 0	0 / 0
B	394	249 / 0	85 / 0	0 / 0	0 / 0	60 / 0	0 / 0
L	384	185 / 0	107 / 0	0 / 0	0 / 0	93 / 0	0 / 0
I	543	294 / 0	129 / 0	0 / 0	0 / 0	120 / 0	0 / 0
K	535	318 / 0	108 / 0	0 / 0	0 / 0	109 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, H, B, L, I, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 72	-101.8	-101.8	0.22 (1)	10.00	
B- N	-125 / 0	-84.3	-84.3	0.21 (1)	6.25	
N- C	-59 / 0	-84.3	-84.3	0.17 (1)	6.25	
C- O	0 / 22	-84.3	-84.3	0.33 (1)	10.00	
D- E	0 / 21	-84.3	-84.3	0.34 (1)	10.00	
E- F	0 / 21	-84.3	-84.3	0.34 (1)	10.00	
F- G	-9 / 0	-84.3	-84.3	0.34 (1)	10.00	
H- G	-190 / 0	0.0	0.0	0.02 (1)	7.81	
B- M	0 / 43	-28.0	-28.0	0.08 (1)	10.00	
M- L	0 / 43	-28.0	-28.0	0.15 (2)	10.00	
L- K	0 / 30	-28.0	-28.0	0.15 (2)	10.00	
K- J	0 / 9	-28.0	-28.0	0.21 (2)	10.00	
J- I	0 / 9	-28.0	-28.0	0.21 (2)	10.00	
I- H	0 / 0	-28.0	-28.0	0.21 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- C	-284 / 0	0.05 (1)
I- G	0 / 10	0.00 (1)
C- K	-56 / 0	0.03 (1)
I- F	-487 / 0	0.08 (1)
K- D	-493 / 0	0.08 (1)
K- F	-33 / 0	0.02 (1)
M- N	-56 / 149	0.03 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(65 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.34 (D-F:1) , BC=0.21 (I-K:2) , WB=0.08 (D-K:1) , SSI=0.22 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (C) (INPUT = 0.80)
JSI METAL= 0.24 (E) (INPUT = 1.00)

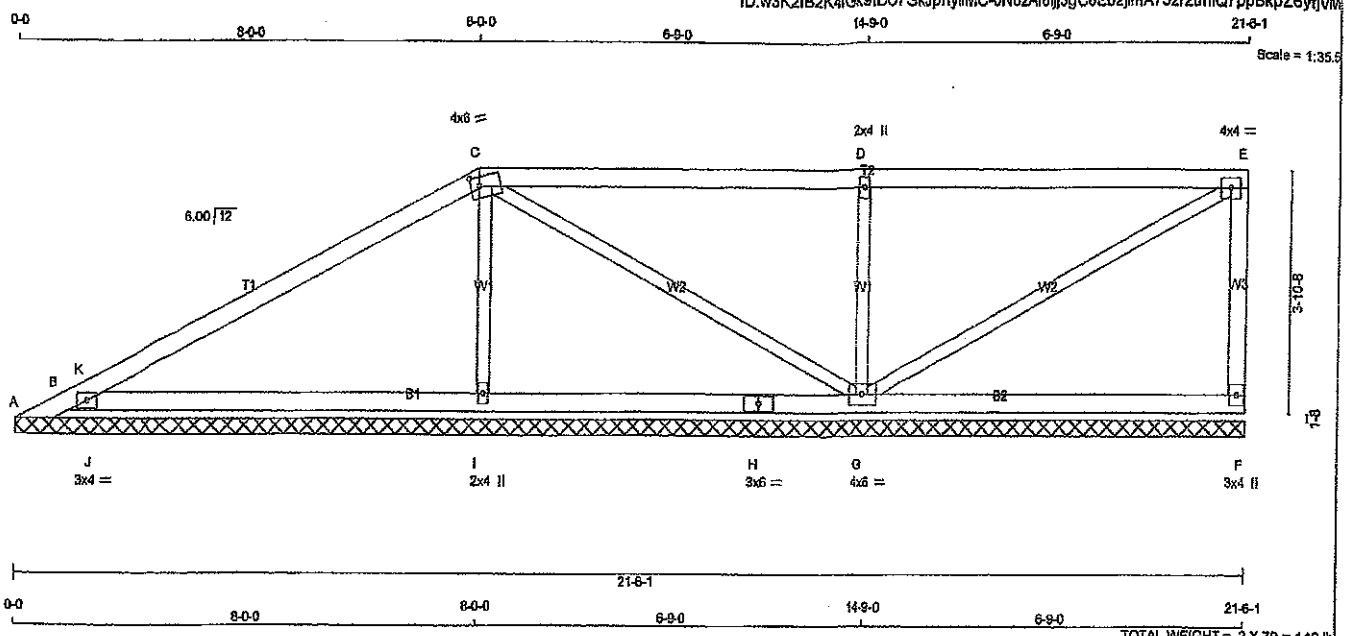
LICENSED PROFESSIONAL ENGINEER
9/11/17
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM4554317
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43854	DRWG NO.
285929	P2	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Fri Sep 08 10:56:47 2017 Page 1
ID:w3K2IB2K4fGk9IDO7SkJphyIMC-0N6zAi8ijj3gC8Eb2jimA73zr2tmIQ7ppBkpZ6yfjvM



TOTAL WEIGHT = 2 X 70 = 140 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

BEARING NOTE: GAP BETWEEN INSIDE OF TOP CHORD BEARING AND FIRST DIAGONAL OR VERTICAL WEB SHALL NOT EXCEED 0.5 INCHES.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW-m	MT20	4.0	6.0	1.75	1.75
D	TMW+w	MT20	2.0	4.0		
E	TMWW-I	MT20	4.0	4.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW1-t	MT20	4.0	6.0		
H	BS-t	MT20	3.0	6.0		
I	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
A	490	0	0	0	-490	21-6-1 (8-7-12)	6-1
F	287	0	287	0	0	21-6-1 (8-7-12)	6-1
B	1065	0	1065	0	0	21-6-1 (8-7-12)	6-1
I	573	0	573	0	0	21-6-1 (8-7-12)	6-1
G	962	0	962	0	0	21-6-1 (8-7-12)	6-1

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): F , I , G

PROVIDE ANCHORAGE AT BEARING JOINT A FOR 490 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	395	0 / -235	0 / -83	0 / 0	0 / 0	0 / -77	0 / 0
F	239	128 / 0	57 / 0	0 / 0	0 / 0	53 / 0	0 / 0
B	845	513 / 0	160 / 0	0 / 0	0 / 0	173 / 0	0 / 0
I	495	296 / 0	139 / 0	0 / 0	0 / 0	120 / 0	0 / 0
G	774	458 / 0	169 / 0	0 / 0	0 / 0	159 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, F, B, I, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 222	-101.8	-101.8	0.67 (1)	0.08 (1)	10.00
B- K	-318 / 0	-84.3	-84.3	0.84 (1)	6.25	
K- C	-111 / 0	-84.3	-84.3	0.45 (1)	6.25	
C- D	-2 / 10	-84.3	-84.3	0.65 (1)	10.00	
D- E	-2 / 10	-84.3	-84.3	0.65 (1)	10.00	
F- E	-210 / 0	0.0	0.0	0.05 (1)	7.81	
B- J	0 / 79	-28.0	-28.0	0.18 (3)	10.00	
J- I	0 / 70	-28.0	-28.0	0.27 (2)	10.00	
I- H	0 / 89	-28.0	-28.0	0.31 (3)	10.00	
H- G	0 / 89	-28.0	-28.0	0.31 (3)	10.00	
G- F	0 / 0	-28.0	-28.0	0.31 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- C	-361 / 0	0.08 (1)
C- G	-89 / 0	0.10 (1)
G- D	-711 / 0	0.17 (1)
G- E	-11 / 2	0.01 (1)
J- K	0 / 493	0.09 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.67 (A-B:1) , BC=0.31 (G-I:3) , WB=0.17 (D-G:1) , SSI=0.41 (A-B:1)

DOL LUMBER=1.60 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (B) (INPUT = 0.90)
JSI METAL= 0.21 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
9/11/17
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 4554417
STRUCTURAL
COMPONENT ONLY

LUL/LUS/LJS/HUS/HHUS/HGUS

Standard and Double-Shear Joist Hangers

This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 258–259.

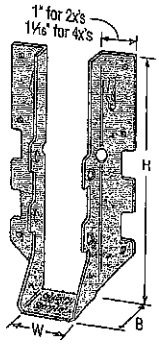
Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 20–24.

Installation:

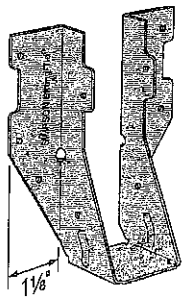
- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails sre specified and 0.77 where 10d nails are specified.

Options:

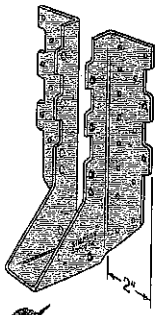
- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on p. 126.



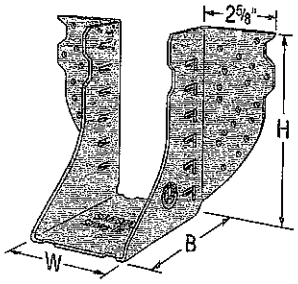
LUS28



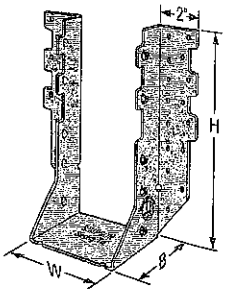
LU26L



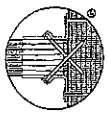
HUS210
(HUS26, HUS28, and HHUS similar)



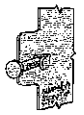
HGUS28-2



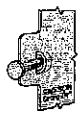
HHUS210-2



Double-Shear Nailing Top View



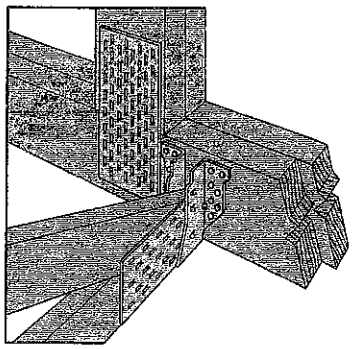
Double-Shear Nailing Side View; Do not bend tab

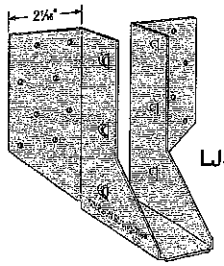


Dome Double-Shear Nailing Side View *(available on some models)*

U.S. Patent 5,603,580

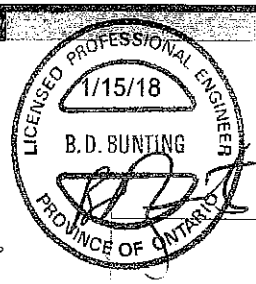
Typical HUS26 Installation with Reduced Heel Height
(Truss Designer to provide fastener quantity for connecting multiple members together)





LJS26DS

LUL/LUS/LJS/HUS/HHUS/HGUS



HHUS/HGUS

See Hanger Options information on pp. 125–127.

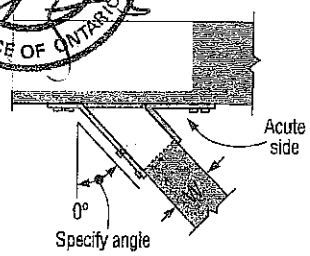
HHUS — Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.62 of the table value
- The joist must be bevel-cut to allow for double-shear nailing

HGUS — Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Joist	Down Resistance	Uplift
W < 2"	Bevel or square cut	0.62 of table value	0.46 of table value
2" < W < 6"	Bevel cut	0.67 of table value	0.41 of table value
2" < W < 6"	Square cut	0.46 of table value	0.41 of table value
W > 6"	Bevel cut	0.75 of table value	0.41 of table value



Top View HHUS Hanger Skewed Right
(joist must be bevel cut)
All joist nails installed on the outside angle *(non-acute side)*.

Standard and Double-Shear Joist Hangers (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.) W	H	B	d_e[3]	Fasteners Header	Joist	Factored Resistance D.Fir-L Uplift (K_D = 1.15) lb. / kN	D.Fir-L Normal (K_D = 1.00) lb. / kN	S-P-F Uplift (K_D = 1.15) lb. / kN	S-P-F Normal (K_D = 1.00) lb. / kN
Single 2x Sizes											
LUS24	18	1⁹⁄₁₆	3⅛	1¾	2¼	(4) 10d	(2) 10d	710 / 3.16	1625 / 7.23	645 / 2.87	1155 / 5.14
LU24L	22	1⁹⁄₁₆	3	1⅝	2¹¹⁄₁₆	(4) 10d	(2) 10d x 1½"	360 / 1.60	1020 / 4.54	320 / 1.42	725 / 3.22
LU26L	22	1⁹⁄₁₆	5	1⅝	4⅝	(6) 10d	(4) 10d x 1½"	720 / 3.20	1605 / 7.14	645 / 2.87	1140 / 5.07
LUS26	18	1⁹⁄₁₆	4¾	1¾	3¾	(4) 10d	(4) 10d	1420 / 6.32	2170 / 9.65	1290 / 5.74	1630 / 7.25
HUS26	16	1⅝	5⅜	3	3¹⁵⁄₁₆	(14) 16d	(6) 16d	2705 / 11.30	4940 / 21.97	2065 / 9.20	3875 / 17.24
LJS26DS	18	1⁹⁄₁₆	5	3½	4⅝	(16) 16d	(6) 16d	2055 / 9.14	4265 / 18.97	1460 / 6.49	4115 / 18.31
HGUS26	12	1⅝	5⅜	5	4⅛	(20) 16d	(8) 16d	2685 / 11.96	6625 / 29.51	2685 / 11.96	5700 / 25.35
LU28L	20	1⁹⁄₁₆	6¾	1⅝	5⅞	(8) 10d	(6) 10d x 1½"	1140 / 5.07	2185 / 9.72	1020 / 4.54	1550 / 6.89
LUS28	18	1⁹⁄₁₆	6⅝	1¾	3¾	(6) 10d	(4) 10d	1420 / 6.32	2520 / 11.21	1290 / 5.74	1790 / 7.96
HUS28	16	1⅝	7¹⁄₁₆	3	6¹⁄₁₆	(22) 16d	(8) 16d	3605 / 16.04	5365 / 23.86	2675 / 11.90	4345 / 19.33
HGUS28	12	1⅝	7⅛	5	6⅛	(36) 16d	(12) 16d	3310 / 14.74	7675 / 34.19	3310 / 14.74	6900 / 30.73
LU210L	20	1⁹⁄₁₆	8	1⅝	7⅝	(10) 10d	(6) 10d x 1½"	1140 / 5.07	2495 / 11.10	1020 / 4.54	1770 / 7.87
LUS210	18	1⁹⁄₁₆	7¹³⁄₁₆	1¾	3⅞	(8) 10d	(4) 10d	1420 / 6.32	2785 / 12.39	1290 / 5.74	2210 / 9.83

1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.
3. d_e is the distance from the bearing seat to the top joist nail.
4. Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-C-N10TRSSCN and/or see installation notes.
5. **Nails:** 16d = 0.162" dia. x 3½" long. See pp. 27–28 for other nail sizes and information.

Face-Mount Hangers

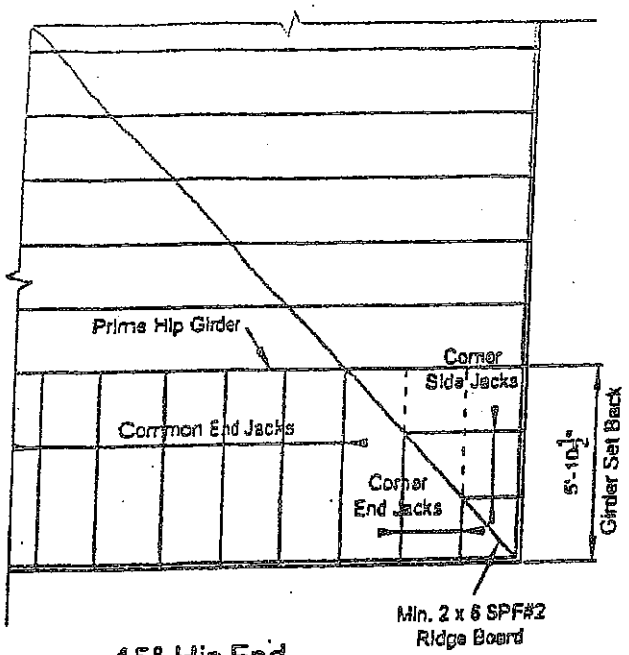
These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

Model No.	Ga.	Dimensions (In.) W	H	B	d_e[3]	Fasteners Header	Joist	Factored Resistance D.Fir-L Uplift (K_D = 1.15) lb. / kN	D.Fir-L Normal (K_D = 1.00) lb. / kN	S-P-F Uplift (K_D = 1.15) lb. / kN	S-P-F Normal (K_D = 1.00) lb. / kN
Double 2x Sizes											
LUS24-2	18	3⅛	3⅛	2	1½	(4) 16d	(2) 16d	835 / 3.71	2020 / 8.99	590 / 2.62	1435 / 6.38
LUS26-2	18	3⅛	4⅞	2	4	(4) 16d	(4) 16d	1720 / 7.65	2595 / 11.54	1545 / 6.87	1920 / 8.54
HHUS26-2	14	3⅝	5⅜	3	3⁵⁄₁₆	(14) 16d	(6) 16d	2850 / 12.68	7335 / 32.63	2065 / 9.20	5205 / 23.15
HGUS26-2	12	3⅝	5⅞	4	4⅛	(20) 16d	(8) 16d	4385 / 19.51	8950 / 39.81	3110 / 13.83	6355 / 28.27
LUS28-2	18	3⅛	7	2	4	(6) 16d	(4) 16d	1720 / 7.65	3325 / 14.79	1545 / 6.87	2575 / 11.45
HHUS28-2	14	3⅝	7⅝	3	6⅛	(22) 16d	(8) 16d	3765 / 16.75	8940 / 39.77	2675 / 11.90	6345 / 28.22
HGUS28-2	12	3⅝	7⅞	4	6⅛	(36) 16d	(12) 16d	6070 / 27.00	12980 / 57.74	4310 / 19.17	9215 / 40.99
LUS210-2	18	3⅛	9	2	6	(8) 16d	(6) 16d	2580 / 11.48	4500 / 20.02	2320 / 10.32	3195 / 14.21
HHUS210-2	14	3⅝	9⅝	3	8	(30) 16d	(10) 16d	4670 / 20.77	9660 / 42.97	4235 / 18.84	7000 / 31.14
HGUS210-2	12	3⅝	9⅞	4	8⅛	(46) 16d	(16) 16d	6840 / 30.43	14015 / 62.34	4855 / 21.60	10270 / 45.69
Triple 2x Sizes											
HGUS26-3	12	4¹⁵⁄₁₆	5½	4	4⅛	(20) 16d	(8) 16d	4385 / 19.51	8950 / 39.81	3110 / 13.83	6355 / 28.27
HGUS28-3	12	4¹⁵⁄₁₆	7¼	4	6⅛	(36) 16d	(12) 16d	6070 / 27.00	12980 / 57.74	4310 / 19.17	9215 / 40.99
HHUS210-3	14	4¹¹⁄₁₆	9	3	7¹⁵⁄₁₆	(30) 16d	(10) 16d	4670 / 20.77	9670 / 43.02	4235 / 18.84	6865 / 30.54
HGUS210-3	12	4¹⁵⁄₁₆	9¼	4	8⅛	(46) 16d	(16) 16d	6840 / 30.43	14645 / 65.14	4855 / 21.60	10400 / 46.26
Quadruple 2x Sizes											
HGUS26-4	12	6⅝	5⅞	4	4⅛	(20) 16d	(8) 16d	4385 / 19.51	8950 / 39.81	3110 / 13.83	6355 / 28.27
HGUS28-4	12	6⅝	7⅞	4	6⅛	(36) 16d	(12) 16d	6070 / 27.00	12980 / 57.74	4310 / 19.17	9215 / 40.99
HHUS210-4	14	6⅛	8⅞	3	7¹⁵⁄₁₆	(30) 16d	(10) 16d	4670 / 20.77	10155 / 45.17	4235 / 18.84	7210 / 32.07
HGUS210-4	12	6⅝	9⅞	4	8⅛	(46) 16d	(16) 16d	6840 / 30.43	14645 / 65.14	4855 / 21.60	10400 / 46.26
HGUS212-4	12	6⅝	10⅝	4	10⅛	(56) 16d	(20) 16d	7640 / 33.98	14995 / 66.70	5425 / 24.13	10645 / 47.35
HGUS214-4	12	6⅝	12⅝	4	11⅛	(66) 16d	(22) 16d	10130 / 45.06	16400 / 72.95	7195 / 32.00	11645 / 51.80
4x Sizes											
LUS46	18	3⅝	4¾	2	3⅞	(4) 16d	(4) 16d	1720 / 7.65	2595 / 11.54	1545 / 6.87	1920 / 8.54
HHUS46	14	3⅝	5¼	3	3¹⁵⁄₁₆	(14) 16d	(6) 16d	2540 / 11.30	7335 / 32.63	2065 / 9.20	5205 / 23.15
HGUS46	12	3⅝	5¼	4	4⅛	(20) 16d	(8) 16d	4385 / 19.51	8950 / 39.81	3110 / 13.83	6355 / 28.27
LUS48	18	3⅝	6¾	2	3⅞	(6) 16d	(4) 16d	1720 / 7.65	3325 / 14.79	1545 / 6.87	2575 / 11.45
HHUS48	14	3⅝	7⅛	3	6⅛	(22) 16d	(8) 16d	3765 / 16.75	8940 / 39.77	2675 / 11.90	6345 / 28.22
HGUS48	12	3⅝	7⅛	4	6⅛	(36) 16d	(12) 16d	6070 / 27.00	12980 / 57.74	4310 / 19.17	9215 / 40.99
LUS410	18	3⅝	8¾	2	5⅝	(8) 16d	(6) 16d	2580 / 11.48	4500 / 20.02	2320 / 10.32	3195 / 14.21
HGUS410	12	3⅝	9	4	8⅛	(46) 16d	(16) 16d	6840 / 30.43	14015 / 62.34	4855 / 21.60	10270 / 45.69
HGUS412	12	3⅝	10⅞	4	10⅛	(56) 16d	(20) 16d	7640 / 33.98	14995 / 66.70	5425 / 24.13	10645 / 47.35
HGUS414	12	3⅝	12⅞	4	11⅛	(66) 16d	(22) 16d	10130 / 45.06	16400 / 72.95	7195 / 32.00	11645 / 51.80

See footnotes on p. 258.

MICRO CITY
ENGINEERING SERVICES INC.
TEL: (519) 287 - 2242



45° Hip End

LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

DESIGN LOAD:

TOP CHORD LIVE LOAD	:	34.8 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD	:	44.8 P.S.F.

DWG NO TAM 3495.14 R1
STRUCTURAL COMPONENT ONLY

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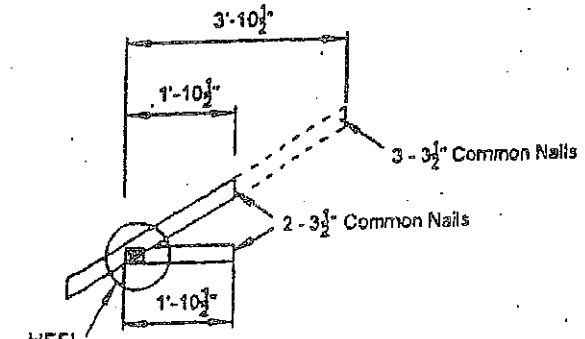
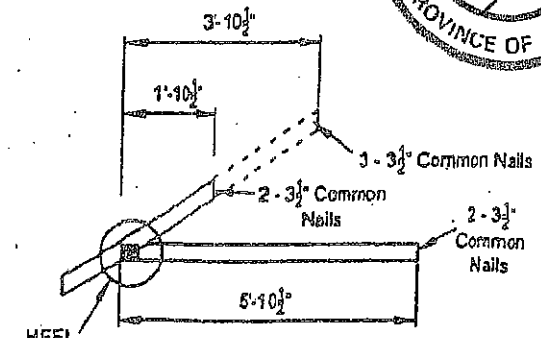
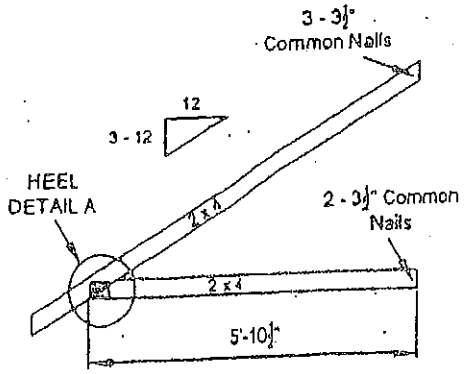


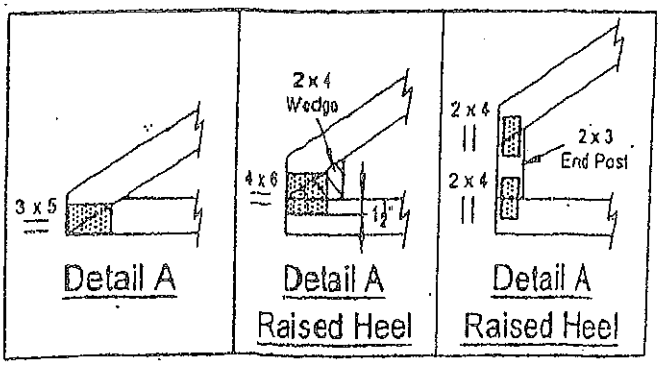
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

MICRO CITY
ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

DESIGN LOAD:

TOP CHORD LIVE LOAD	: 40.5 P.S.F.
TOP CHORD DEAD LOAD	: 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	: 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	: 7.0 P.S.F.
TOTAL LOAD	: 50.5 P.S.F.

DWG NO TAM 3504.1
STRUCTURAL
COMPONENT ONLY

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S. KATSOULAKOS
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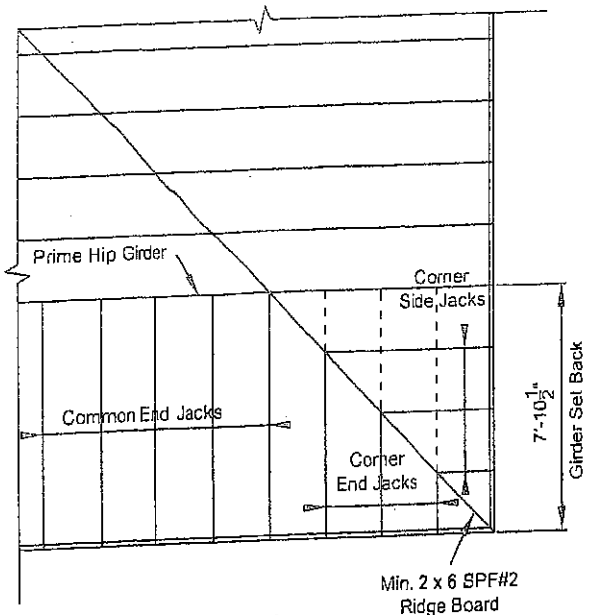
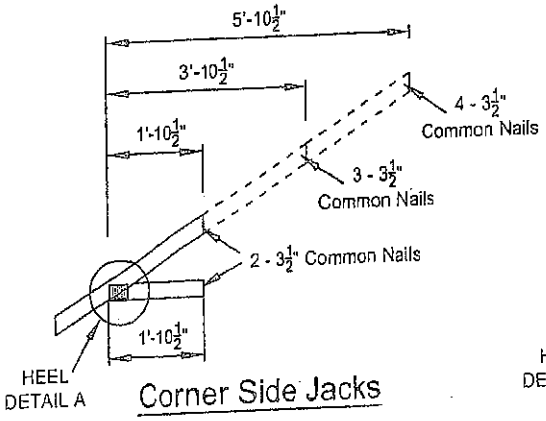
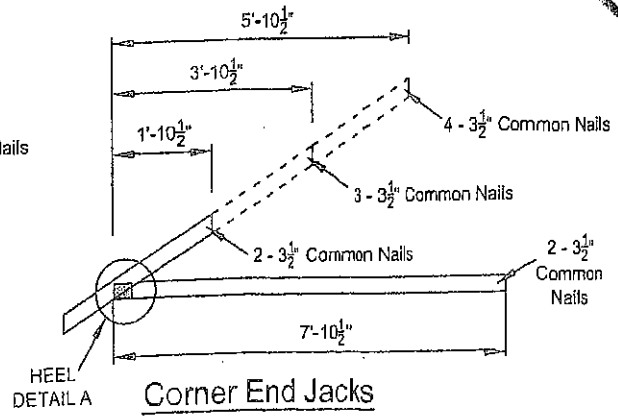


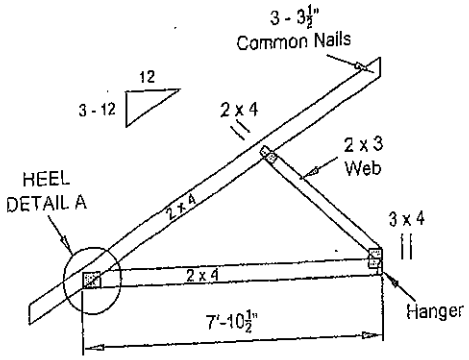
45° Hip End



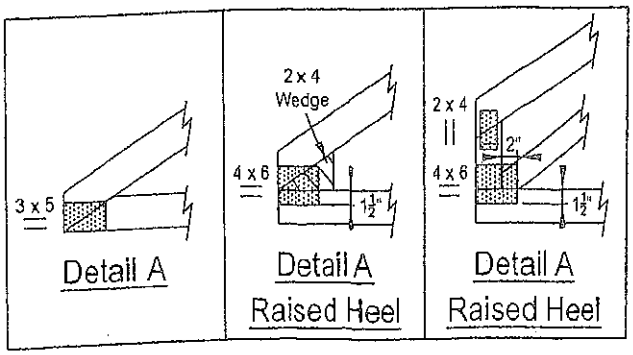
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

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TEL: (519) 287 - 2242

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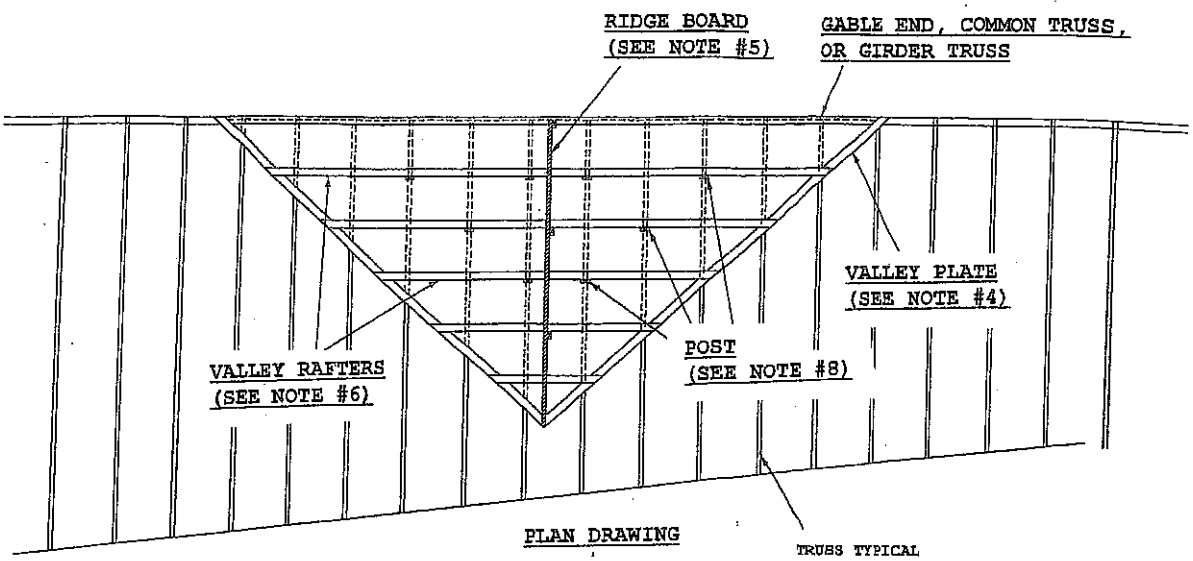
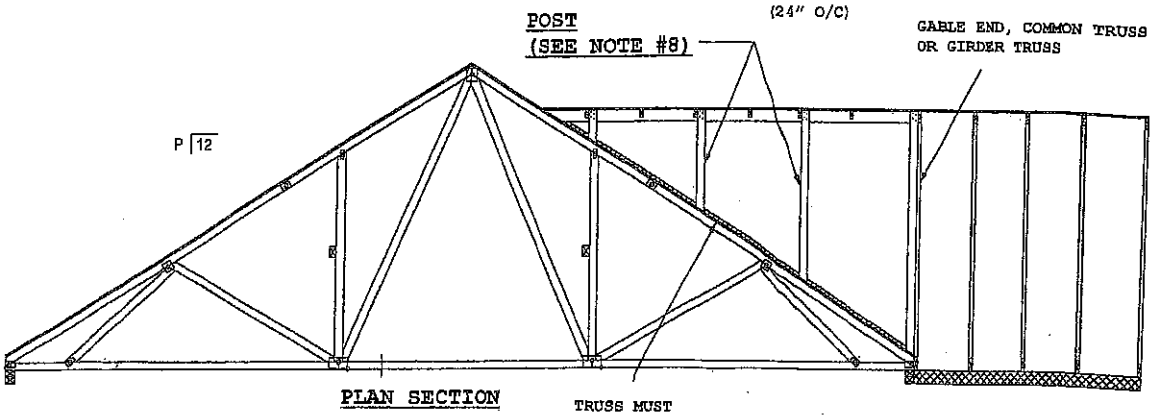
CONVENTIONAL VALLEY FRAMING DETAIL



PLAN DRAWING

TRUSS TYPICAL
(24" O/C)



PLAN SECTION

TRUSS MUST
BE SHEATHED

GENERAL SPECIFICATIONS:

(1) WITH THE BASE TRUSSES ERECTED (INSTALLED), APPLY SHEATHING TOP CHORD OF SUPPORTING (BASE) TRUSSES.
(2) BRACE BOTTOM CHORD AND WEB MEMBERS AS PER PRE-ENGINEERED TRUSS DESIGNS.
(3) DEFINE VALLEY RIDGE BY RUNNING A LEVEL STRING FROM THE INTERSECTING RIDGE OF THE (a) GABLE END, (b) GIRDER TRUSS OR (c) COMMON TRUSS TO THE ROOF SHEATHING.
(4) INSTALL 2 X 6 VALLEY PLATES ON FLAT. FASTEN TO EACH SUPPORTING TRUSS WITH (2) 16d (3.5" X 0.131") NAILS.
(5) SET A 2 X 6 #2 RIDGE BOARD (MAX. 10'-0" RIDGE) OR 2 X 8 #2 SPF RIDGE BOARD (MAX. 20'-0" RIDGE). SUPPORT RIDGE BOARD WITH 2 X 4 POSTS SPACED 48" O/C. BEVEL BOTTOM OF POST TO SET EVENLY ON THE SHEATHING. FASTEN POST TO RIDGE WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST TO ROOF SHEATHING WITH (3) 10d (3" X 0.131") TOE-NAILS.
(6) FRAME VALLEY RAFTERS FROM VALLEY PLATE TO RIDGE BOARD. MAXIMUM RAFTER SPACING IS 24" O/C. FASTEN VALLEY RAFTER TO RIDGE BEAM WITH (3) 16d (3.5" X 0.131") TOE-NAILS. FASTEN VALLEY RAFTER TO VALLEY PLATE WITH (3) 16d (3.5" X 0.131") TOE-NAILS.
(7) SUPPORT THE VALLEY RAFTERS WITH 2 X 4 POSTS AT 48" O/C (OR LESS) ALONG EACH RAFTER. INSTALL POSTS IN A STAGGERED PATTERN AS SHOWN ON PLAN DRAWING. ALIGN POSTS WITH TRUSSES BELOW. FASTEN VALLEY RAFTER TO POST WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST THROUGH SHEATHING TO SOPPORTING TRUSSES WITH (2) 16d (3.5" X 0.131") NAILS.
(8) POSTS SHALL BE 2 X 4 #2 SPF OR BETTER. POSTS EXCEEDING 75" IN HEIGHT SHALL BE INCREASED TO 4 X 4 #2 SPF, OR BETTER, OR BE PRE-ASSEMBLED TWO (2) PLY 2 X 4 #2 SPF OR BETTER FASTENED TOGETHER WITH 2 ROWS OF 10d (3" X 0.131") NAILS AT 6" O/C.
(9) MAINTAIN A MINIMUM 3/4" LUMBER EDGE DISTANCE WHEN NAILING. NAIL SPACING SHOULD APPROXIMATE A MINIMUM 1-3/4" O/C OR MORE UNLESS NOTED OTHERWISE. ALL CONSTRUCTION TO CONFORM TO ONTARIO BUILDING CODE (CURRENT ADDITION) AT ALL TIMES.

NOTES:

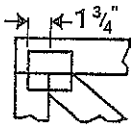
(10) 48" O/C (MAXIMUM POST SPACING.
(11) ROOF LIVE LOAD = 34.8 PSF (MAX.)
(12) ROOF DEAD LOAD = 10.0 PSF (MAX.)
(13) PART 9 APPLICATION ONLY (ONTARIO BUILDING CODE)
(14) PART 4 APPLICATION ONLY (ONTARIO BUILDING CODE) WITH APPROVED REVIEW BY LICENSED PROFESSIONAL ENGINEER.
(15) BASE TRUSS SPACING (24" O/C MAX.)
(16) ALL PRE-ENGINEERED BASE TRUSS COMPONENTS TO BE SEALED BY LICENSED PROFESSIONAL ENGINEER AND THIS DETAIL TO BE VERIFIED AND APPROVED BY SAME WHEN RIDGE BOARD LENGTH EXCEEDS 12'-0".
(17) ALL BASE TRUSSES: P = 4 (4/12) - MINIMUM.
(18) ALL VALLEY RAFTERS: P = 4 (4/12) - MINIMUM.

LICENSED PROFESSIONAL ENGINEER
3-574
S. KATSOULAKOS
PROVINCE OF ONTARIO

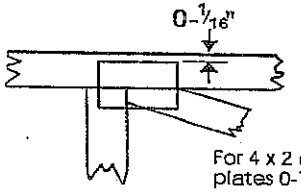
DWG NO TAM 6305.14
STRUCTURAL
COMPONENT ONLY

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

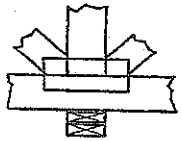
4 X 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

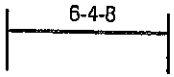
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses

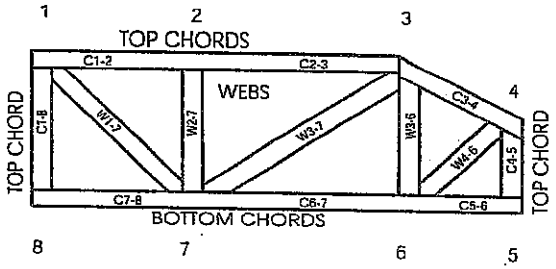
DSB-89: Design Standard for Bracing.

BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

MiTek

POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MII-7473C rev. 10-'08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

Micro City Engineering Services Inc.
(BCIN: 26064; FIRM BCIN: 29991)
RR #1, Po Box 61
Glencoe, Ontario
N0L 1M0
(519) 287 - 2242; Fax: (519) 287 - 5750 (Call)

LICENSED PROFESSIONAL ENGINEER
1-2014
S. KATSOULAKOS
PROVINCE OF ONTARIO
DWG NO T&M 5[illegible]7[illegible]4
STRUCTURAL
COMPONENT ONLY

Responsibilities:

Micro City Engineering Services is responsible for the design of trusses as individual components.

It is the responsibilities of others to ascertain that the design loads utilized on this (these) drawing(s) meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdiction over such decisions.

All dimensions are to be verified by the owner, contractor, architect, or other authority having input over such decisions prior to truss component manufacture. At no time shall Micro City Engineering Services Inc. or its employees be responsible for dimension errors.

Micro City Engineering Services Inc. bears no responsibility for the erection of any truss components. Persons erecting truss components are cautioned to seek professional advice regarding temporary and permanent bracing systems and to be **totally** familiar with all aspects of truss erection prior to proceeding on **any** truss component erection job. Any bracing shown on Micro City Engineering Services Inc. or Tamarack Roof Trusses Inc. sealed or unsealed truss component drawings is specified for the single truss component in question and is identified as an integral part of the design for that particular truss component but is **not** meant to represent the only required bracing for that particular truss component when installed as a component in a series of truss components in a roof truss system.

It is the truss manufacturer's responsibility to ensure that trusses are manufactured in accordance with Micro City Engineering Services Inc. specifications outlined below:

SPECIFICATIONS:

Truss components sealed by Micro City Engineering Services Inc. must conform to the relevant sections of the current Building Code of Ontario and Canada (Part 4 or Part 9) or the current Farm Building Code of Canada in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the Truss Plate Institute of Canada (TPIC). All unit lumber and nailing stresses identified on truss component design drawings and/or used in the design of individual truss components shall conform to the current CSA Wood Design standard identified in the current Building Code and TPIC Design Standards.

The lumber used to manufacture any truss component is to conform to the specified size and grade identified on the truss drawing.

The lumber used in the manufacture of any truss component is not to exceed 19% during its service use unless specifically noted on the truss drawing.

The lumber used in the manufacture of any truss component is not to be treated with any chemicals during its service life unless specifically noted on the truss drawing.

Connector plates shall be applied to both faces of the truss component at each joint and shall be positioned exactly as specified.

The top chord of any truss component is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the sealed truss component drawing but not exceeding 24" o/c (Part 9 design) and not exceeding 48" o/c (Part 4 or Agricultural design).

When a truss component is to be installed with no rigid ceiling attached directly to the bottom chord, then the bottom chord is to be laterally braced at intervals not exceeding 3m (or 10'-0").

All sealed or unsealed truss component drawings provided by Micro City Engineering Services Inc. Or Tamarack Roof Trusses Inc. should be read in conjunction with the following:

Warning-Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473C rev 10-'08 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer - not the truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component Safety Information available from the Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA, 22314.