ASPHALT SHINGLES
12"FINISH O.H.
R.T.M.C
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:

HGUS26-2 -(XX)
HGUS26-3 -(S)
LJS26DS -(V)
LUS24 -(O)

8/12 PITCHES (TYP.)
UNLESS NOTED

DENOTES:

H2 -1'-6"-0 RAISED CEILING
H3 -1'-9"-0 RAISED CEILING

ALL B- 2-2X10 BEAMS (FLUSH)

T-170534

DENOTES:
CONVENTIONAL
FRAMING

HIGHGROVE-5E
EL:3A - UNIT1

285813

HIGHGROVE-4
EL:3 UNIT2

285814

HIGHGROVE-4
EL:3 (REV) UNIT3

285815

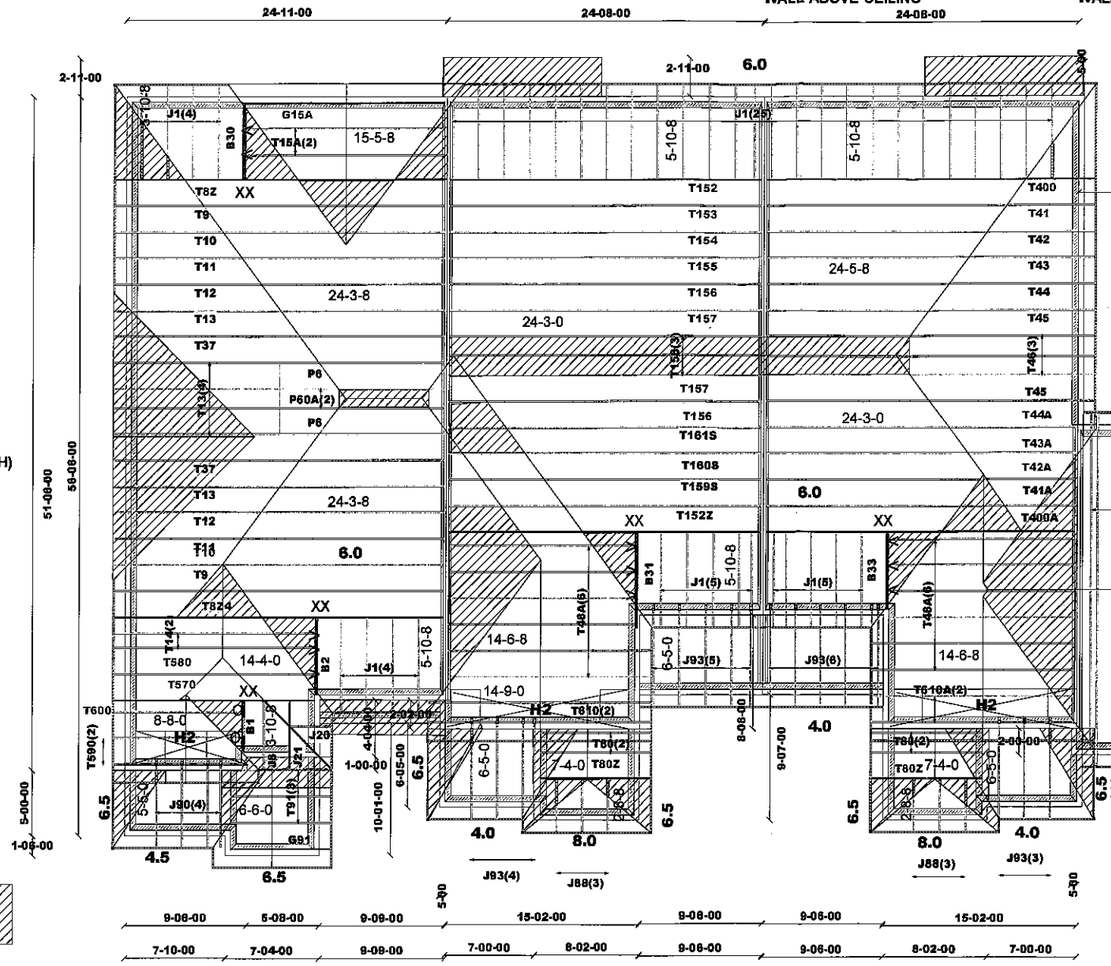
HIGHGROVE-6E
EL:3 UNIT4

285817

C OF WOOD PARTY WALL ABOVE CEILING

C OF WOOD PARTY WALL ABOVE CEILING

C OF WOOD PARTY WALL ABOVE CEILING



ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION
ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"o.c.
WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.
POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE
BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN LOADS:

SNOW LOAD 25.6 PSF
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

M10,997

TAMARACK LUMBER INC. — ALPA LUMBER GROUP

Job Track: 43954	Builder / Location: GREENPARK HOMES / WATERDOWN	Model / Elevation: BLOCK212/ UNIT1 TO4 (REVISED)	Mitek ver 7.5.0
Layout ID: 285812	Project: RUSSEL GARDENS PHASE 2		
Plan Log: 93446	Date: 4/19/2018 Designer: sonny		

TAMARACK LUMBER INC
ALPA LUMBER GROUP

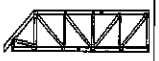
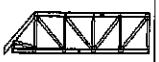
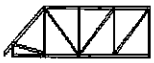
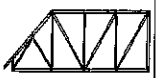
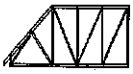
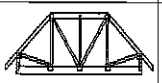
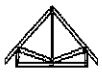
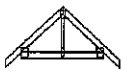
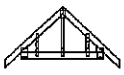
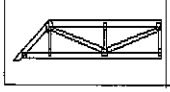
Delivery Shiplist

DATE	04/19/18
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 285813 LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH SUB-BUILDER:
MODEL: HIGHGROVE 5E ELEVATION: 3A- UNIT1

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T8Z HALF HIP	8.00 0.00	24-03-08	04-01-04	2 X 6	2 X 6	01-03-08 00-00-00	01-04-13 04-01-04	143.26 88.34		
	1	T8Z4 HALF HIP	8.00 0.00	24-03-08	04-01-04	2 X 6	2 X 6	01-03-08 00-00-00	01-04-13 04-01-04	143.26 88.34		
	2	T9 HALF HIP	8.00 0.00	24-03-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-01-04	197.10 123.66		
	2	T10 HALF HIP	8.00 0.00	24-03-08	06-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 06-01-04	212.66 133.34		
	2	T11 HALF HIP	8.00 0.00	24-03-08	07-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 07-01-04	225.56 142.68		
	2	T12 HALF HIP	8.00 0.00	24-03-08	08-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 08-01-04	234.64 146.34		
	6	T13 HALF HIP	8.00 0.00	24-03-08	09-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 09-01-04	737.28 454.98		
	2	T37 HALF HIP	8.00 0.00	24-03-08	10-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 10-01-04	257.54 159.34		
	2	T15A COMMON	8.00 0.00	15-05-08	06-07-08	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	129.00 82.66		
	1	G15A COMMON	8.00 0.00	15-05-08	06-07-08	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	67.52 44.33		
	2	T14 COMMON	8.00 0.00	14-04-00	06-02-02	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	123.52 78.00		
	1	T570 HIP GIRDER	8.00 0.00	14-04-00	03-11-13	2 X 4	2 X 6	01-03-08 01-03-08	01-04-13 01-04-13	70.63 46.34		
	1	T580 HIP	8.00 0.00	14-04-00	05-03-13	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	62.73 39.33		
	2	T590 ROOF	8.00 8.00	08-08-00	04-03-08	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	83.72 58.68		
	1	T600 COMMON	8.00 0.00	08-08-00	04-03-08	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	37.48 23.67		
	3	T91 COMMON	6.50 0.00	06-06-00	02-04-09	2 X 4	2 X 4	01-03-08 01-03-08	00-07-07 00-07-07	66.60 45.00		
	1	G91 COMMON	6.50 0.00	06-06-00	02-04-09	2 X 4	2 X 4	01-03-08 01-03-08	00-07-07 00-07-07	23.39 16.00		
	2	P6 PIGGYBACK	8.00 0.00	11-11-15	01-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 01-10-08	80.22 51.34		

TAMARACK LUMBER INC
ALPA LUMBER GROUP

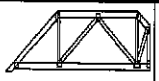
Delivery Shiplist

DATE	04/19/18
SALES REP	Mario

JOB TRACK: 43954 **LAYOUT ID:** 285813 **LOCATION:**
BUILDER: GREENPARK - RUSSELL GARDENS PH **SUB-BUILDER:**
MODEL: HIGHGROVE 5E **ELEVATION:** 3A- UNIT1

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	2	P60A PIGGYBACK	8.00 0.00	11-11-15	02-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 02-01-12	87.08 58.00		
	8	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	134.32 85.36		
	1	J8 JACK-OPEN	8.00 0.00	03-10-08	03-11-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 03-11-13	12.81 8.00		
	1	J20 JACK-OPEN	8.00 0.00	01-10-08	02-07-02	2 X 4	2 X 4	01-03-08 -00-01-01	01-04-13 00-03-08	9.17 5.67		
	1	J21 JACK-OPEN	8.00 0.00	03-10-08	02-07-02	2 X 4	2 X 4	01-03-08 -02-01-01	01-04-13 00-03-08	12.97 8.00		
	4	J90 MONOPITCH	4.50 0.00	05-05-00	02-05-00	2 X 4	2 X 4	01-03-08 00-00-00	00-04-10 02-05-00	57.88 37.32		

TOTAL # TRUSS= **51.00** TOTAL BFT OF ALL TRUSSES= **2024.72** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3210.34** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	HGUS26-2	
5	Hangers	LJS26DS	
2	Hangers	LUS24	

TOTAL # ITEMS= **10.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

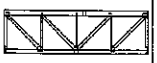
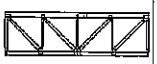
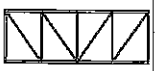
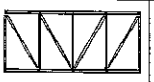
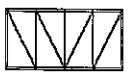
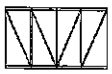
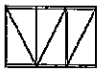
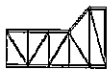
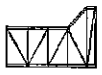
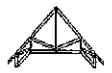
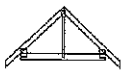
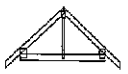
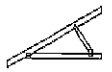
Delivery Shiplist

DATE	09/06/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 285814 LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH 2 SUB-BUILDER:
MODEL: HIGHGROVE 4 ELEVATION: 3-UNIT2

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T152 FLAT GIRDER	0.00 0.00	24-03-00	04-01-04	2 X 6	2 X 6	00-00-00 00-00-00	04-01-04 04-01-04	138.69 88.67		
	1	T152Z FLAT GIRDER	0.00 0.00	24-03-00	04-01-04	2 X 6	2 X 6	00-00-00 00-00-00	04-01-04 04-01-04	138.69 88.67		
	1	T153 FLAT	0.00 0.00	24-03-00	05-01-04	2 X 4	2 X 4	00-00-00 00-00-00	05-01-04 05-01-04	100.52 63.50		
	1	T154 FLAT	0.00 0.00	24-03-00	06-01-04	2 X 4	2 X 4	00-00-00 00-00-00	06-01-04 06-01-04	107.38 66.33		
	1	T155 FLAT	0.00 0.00	24-03-00	07-01-04	2 X 4	2 X 4	00-00-00 00-00-00	07-01-04 07-01-04	114.46 71.17		
	2	T156 FLAT	0.00 0.00	24-03-00	08-01-04	2 X 4	2 X 4	00-00-00 00-00-00	08-01-04 08-01-04	243.40 152.00		
	2	T157 FLAT	0.00 0.00	24-03-00	09-01-04	2 X 4	2 X 4	00-00-00 00-00-00	09-01-04 09-01-04	266.88 162.66		
	3	T158 FLAT	0.00 0.00	24-03-00	10-01-04	2 X 4	2 X 4	00-00-00 00-00-00	10-01-04 10-01-04	421.80 258.00		
	1	T159S ROOF	8.00 0.00	24-03-00	08-04-04	2 X 4	2 X 4	00-00-00 00-00-00	05-01-04 08-04-04	116.49 74.00		
	1	T160S ROOF	8.00 0.00	24-03-00	09-04-04	2 X 4	2 X 4	00-00-00 00-00-00	06-01-04 09-04-04	125.57 79.83		
	1	T161S ROOF	8.00 0.00	24-03-00	10-04-04	2 X 4	2 X 4	00-00-00 00-00-00	07-01-04 10-04-04	134.98 85.17		
	2	T610 ROOF	8.00 8.00	14-09-00	06-03-13	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	125.70 83.66		
	6	T48A COMMON	8.00 0.00	14-06-08	06-03-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	366.42 229.98		
	2	T80 COMMON	6.50 0.00	07-04-00	02-07-04	2 X 4	2 X 4	01-03-08 01-03-08	00-07-07 00-07-07	48.84 31.34		
	1	T80Z COMMON	6.50 0.00	07-04-00	02-07-04	2 X 4	2 X 4	01-03-08 01-03-08	00-07-07 00-07-07	24.42 15.67		
	18	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	302.22 192.06		
	3	J88 JACK-OPEN	8.00 0.00	02-08-08	02-07-05	2 X 4	2 X 4	01-03-08 00-00-00	00-09-10 02-07-05	27.57 18.00		
	9	J93 MONOPITCH	4.00 0.00	06-05-00	02-05-10	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-05-10	163.44 105.03		

TOTAL # TRUSS= **56.00** TOTAL BFT OF ALL TRUSSES= **1865.74** BFT. TOTAL WEIGHT OF ALL TRUSSES= **2967.47** LBS.

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	09/06/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 285814 LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH 2 SUB-BUILDER:
MODEL: HIGHGROVE 4 ELEVATION: 3-UNIT2

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	HGUS26-2	
3	Hangers	LJS26DS	

TOTAL # ITEMS= **4.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

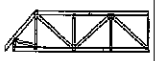
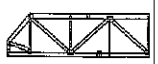
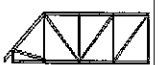
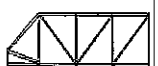
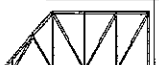
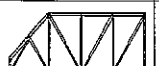
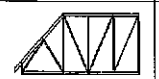
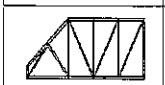
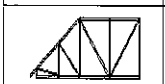
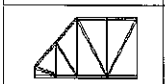
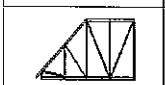
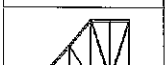
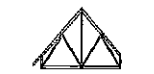
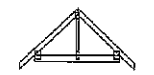
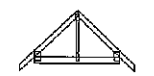
Delivery Shiplist

DATE	09/06/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 285815 LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH 2 SUB-BUILDER:
MODEL: HIGHGROVE 4 ELEVATION: 3 (REV)- UNIT3

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	1	T400 HALF HIP	8.00 0.00	24-05-08	04-01-04	2 X 6	2 X 6	01-03-08 00-00-00	01-04-13 04-01-04	136.90 85.33		
	1	T400A HALF HIP	8.00 0.00	24-03-00	04-01-04	2 X 6	2 X 6	00-00-00 00-00-00	01-06-08 04-01-04	133.35 83.33		
	1	T41 HALF HIP	8.00 0.00	24-05-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-01-04	99.05 61.83		
	1	T41A HALF HIP	8.00 0.00	24-03-00	05-01-04	2 X 4	2 X 4	00-00-00 00-00-00	01-06-08 05-01-04	96.77 61.17		
	1	T42 HALF HIP	8.00 0.00	24-05-08	06-01-04	2 X 4	2 X 4	00-00-00 00-00-00	01-04-13 06-01-04	106.88 66.67		
	1	T42A HALF HIP	8.00 0.00	24-03-00	06-01-04	2 X 4	2 X 4	00-00-00 00-00-00	01-06-08 06-01-04	104.62 66.00		
	1	T43 HALF HIP	8.00 0.00	24-05-08	07-01-04	2 X 4	2 X 4	00-00-00 00-00-00	01-04-13 07-01-04	113.31 71.34		
	1	T43A HALF HIP	8.00 0.00	24-03-00	07-01-04	2 X 4	2 X 4	00-00-00 00-00-00	01-06-08 07-01-04	111.05 70.67		
	1	T44 HALF HIP	8.00 0.00	24-05-08	08-01-04	2 X 4	2 X 4	00-00-00 00-00-00	01-04-13 08-01-04	117.95 73.17		
	1	T44A HALF HIP	8.00 0.00	24-03-00	08-01-04	2 X 4	2 X 4	00-00-00 00-00-00	01-06-08 08-01-04	115.66 73.17		
	2	T45 HALF HIP	8.00 0.00	24-05-08	09-01-04	2 X 4	2 X 4	00-00-00 00-00-00	01-04-13 09-01-04	246.98 151.66		
	3	T46 HALF HIP	8.00 0.00	24-05-08	10-01-04	2 X 4	2 X 4	00-00-00 00-00-00	01-04-13 10-01-04	388.08 239.01		
	2	T610A ROOF	8.00 8.00	14-06-08	06-03-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	119.86 81.00		
	6	T48A COMMON	8.00 0.00	14-06-08	06-03-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	366.42 229.98		
	2	T80 COMMON	6.50 0.00	07-04-00	02-07-04	2 X 4	2 X 4	01-03-08 01-03-08	00-07-07 00-07-07	48.84 31.34		
	1	T80Z COMMON	6.50 0.00	07-04-00	02-07-04	2 X 4	2 X 4	01-03-08 01-03-08	00-07-07 00-07-07	24.42 15.67		
	17	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	285.43 181.39		
	3	J88 JACK-OPEN	8.00 0.00	02-08-08	02-07-05	2 X 4	2 X 4	01-03-08 00-00-00	00-09-10 02-07-05	27.57 18.00		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

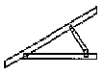
Delivery Shiplist

DATE	09/06/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 285815 LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH2 SUB-BUILDER:
MODEL: HIGHGROVE 4 ELEVATION: 3 (REV)- UNIT3

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	9	J93 MONOPITCH	4.00 0.00	06-05-00	02-05-10	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-05-10	163.44 105.03		

TOTAL # TRUSS= **55.00** TOTAL BFT OF ALL TRUSSES= **1765.76** BFT. TOTAL WEIGHT OF ALL TRUSSES= **2806.58** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	HGUS26-2	
3	Hangers	LJS26DS	

TOTAL # ITEMS= **4.00**

TAMARACK LUMBER INC
ALPA LUMBER GROUP

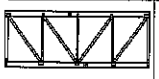
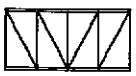
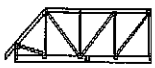
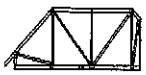
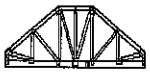
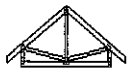
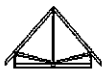
Delivery Shiplist

DATE	04/18/18
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 285817 LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH SUB-BUILDER:
MODEL: HIGHGROVE 6E ELEVATION: 3-UNIT4

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T162 FLAT GIRDER	0.00 0.00	24-04-00	05-03-13	2 X 6	2 X 6	00-00-00 00-00-00	05-03-13 05-03-13	150.65 93.33		
	1	T163 FLAT	0.00 0.00	24-04-00	06-07-13	2 X 4	2 X 4	00-00-00 00-00-00	06-07-13 06-07-13	111.46 71.17		
	11	T164 COMMON	6.00 0.00	24-04-00	07-03-00	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	1056.66 658.13		
	1	T165A HALF HIP	8.00 0.00	18-09-00	04-01-04	2 X 4	2 X 6	01-03-08 00-00-00	01-04-13 04-01-04	87.85 54.00		
	1	T166A HIP	8.00 0.00	18-09-00	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 04-02-13	82.38 51.83		
	1	T167A HIP	8.00 0.00	18-09-00	06-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 04-02-13	88.59 55.50		
	1	T168A HIP	8.00 0.00	18-09-00	07-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 04-02-13	86.29 54.67		
	2	T169A COMMON	8.00 0.00	18-09-00	07-08-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-13	154.04 98.00		
	1 3 Ply	T170A COMMON	8.00 0.00	18-09-00	07-08-13	2 X 6	2 X 6	00-00-00 00-00-00	01-04-13 01-06-13	360.84 219.99		
	1	T171 HIP GIRDER	6.00 0.00	19-01-00	03-11-13	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	75.39 47.67		
	1 3 Ply	T172 HIP GIRDER	6.00 0.00	19-01-00	04-09-02	2 X 4	2 X 6	00-00-00 00-00-00	01-02-00 01-02-00	318.21 210.00		
	4	T173A COMMON	8.00 0.00	11-01-00	06-06-10	2 X 4	2 X 4	01-05-00 00-00-00	01-05-13 04-02-13	210.28 135.32		
	1	T174A COMMON	8.00 0.00	08-09-00	04-09-10	2 X 4	2 X 4	01-03-08 00-00-00	01-03-08 02-05-13	37.90 25.00		
	1	G174A GABLE	8.00 0.00	08-09-00	04-09-10	2 X 4	2 X 4	01-03-08 00-00-00	01-03-08 02-05-13	36.85 24.50		
	2	T175 COMMON	6.00 0.00	08-04-00	03-03-00	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	67.44 44.34		
	1	T175Z GABLE	6.00 0.00	08-04-00	03-03-00	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	30.22 20.17		
	1	T176 COMMON	8.00 0.00	12-00-00	05-04-13	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	49.42 29.83		
	1	G176 GABLE	8.00 0.00	12-00-00	05-04-13	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	49.38 30.83		

TAMARACK LUMBER INC
ALPA LUMBER GROUP

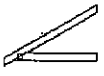
Delivery Shiplist

DATE	04/18/18
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 285817 LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH SUB-BUILDER:
MODEL: HIGHGROVE 6E ELEVATION: 3-UNIT4

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	5	T177TCX MONOPITCH	8.00 0.00	13-06-08	10-05-02	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 10-05-02	373.50 235.85		
	3	T178TCX MONOPITCH	8.00 0.00	09-00-08	12-08-02	2 X 4	2 X 4	00-00-00 00-00-00	06-07-13 12-08-02	233.52 147.00		
	2	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	33.58 21.34		
	1 2 Ply	J1Z JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	33.58 21.34		
	4	J7 JACK-OPEN	8.00 0.00	02-09-04	03-03-00	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 03-03-00	40.16 26.68		
	5	J8 JACK-OPEN	8.00 0.00	03-10-08	03-11-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 03-11-13	64.05 40.00		
	8	J10 JACK-OPEN	8.00 0.00	05-10-08	05-03-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-03-13	142.64 85.36		
	5	J10A JACK-OPEN	8.00 0.00	05-07-08	05-03-13	2 X 4	2 X 4	00-00-00 00-00-00	01-06-13 05-03-13	78.10 50.00		
	6	J55 JACK	4.00 0.00	06-03-00	02-04-15	2 X 4	2 X 4	00-09-08 00-00-00	00-03-15 02-04-15	94.20 60.00		

TOTAL # TRUSS= **77.00** TOTAL BFT OF ALL TRUSSES= **2611.85** BFT. TOTAL WEIGHT OF ALL TRUSSES= **4147.18** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	HGUS26-3	
9	Hangers	LUS24	
12	Hangers	LJS26DS	
2	Hangers	HGUS26-2	

TOTAL # ITEMS= **24.00**

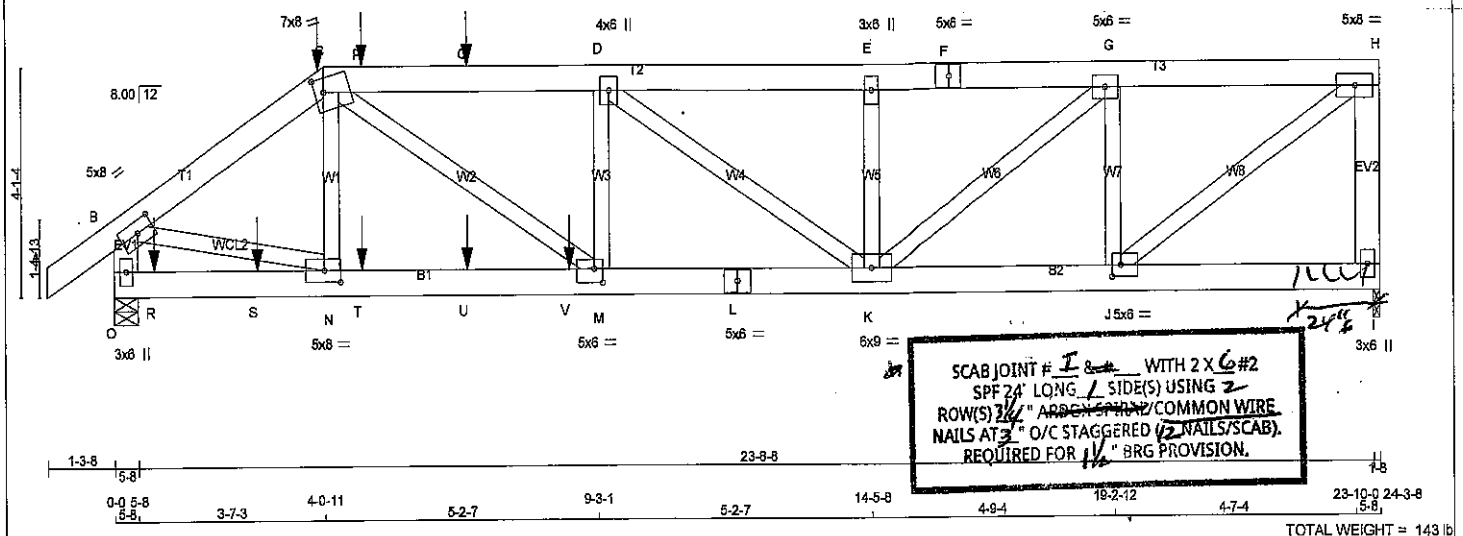
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285301	T8Z	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Wed May 16 08:35:52 2018 Page 1
ID:PFehc1Ba2cOYemTQTAYm6Oyi1EX-_W?aRIBDRIv9cIkSsFt_uHxkl5OtRMl3fvMZu8zG_tL

-1-3-8 0-0 5-8 4-0-11 9-3-1 14-5-8 19-2-12 23-10-0 24-3-8
1-3-8 5-8 3-7-3 5-2-7 5-2-7 4-9-4 4-7-4 5-8

Scale = 1:41.5



SCAB JOINT # I & ___ WITH 2 X 6 #2 SPF 24" LONG 1 SIDE(S) USING 2 ROW(S) 12" ~~ARROW STRAP~~/COMMON WIRE NAILS AT 3" O/C STAGGERED (12 NAILS/SCAB). REQUIRED FOR 1½" BRG PROVISION.

1-3-8 23-8-8 1-8
5-8

0-0 5-8 4-0-11 9-3-1 14-5-8 19-2-12 23-10-0 24-3-8
5-8 3-7-3 5-2-7 5-2-7 4-9-4 4-7-4 5-8

TOTAL WEIGHT = 143 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
I - H	2x6	DRY	No.2	SPF
O - B	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	5.0	6.0	2.50	3.75
C	TTWW-m	MT20	7.0	8.0	3.00	2.00
D	TMWW+t	MT20	4.0	6.0		
E	TMW+w	MT20	3.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMWW-t	MT20	5.0	6.0		
H	TMW-t	MT20	5.0	8.0		
I	BMV1+p	MT20	3.0	8.0		
J	BMWW-t	MT20	5.0	6.0	2.50	2.00
K	BMWWW-t	MT20	6.0	9.0		
L	BS-t	MT20	5.0	6.0		
M	BMWW-t	MT20	5.0	6.0	3.00	2.00
N	BMWW-t	MT20	5.0	8.0	2.50	3.75
O	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 244.5 lbs FACTORED DOWN AT 4-0-11, AND 113.4 lbs FACTORED DOWN AT 4-9-4, AND 101.4 lbs FACTORED DOWN AT 6-9-4 ON TOP CHORD, AND 75.5 lbs FACTORED DOWN AT 9-4, 69.9 lbs FACTORED DOWN AT 2-9-4, 69.9 lbs FACTORED DOWN AT 4-9-4, AND 69.9 lbs FACTORED DOWN AT 6-9-4, AND 1157.0 lbs FACTORED DOWN AT 8-8-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
5/16
S. KATSOULAKOS
PROVINCE OF ONTARIO

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REORD BRG IN-SX
I	1894	0	1894	0	0	1-8	1-8
O	2731	0	2731	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1548	871 / 0	345 / 0	0 / 0	0 / 0	332 / 0	0 / 0
O	2224	1265 / 0	466 / 0	0 / 0	0 / 0	473 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.73 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 33	-84.3	-84.3	0.07 (1)	10.00
B- C	-3245 / 0	-84.3	-84.3	0.19 (1)	4.53
C- P	-4458 / 0	-84.3	-84.3	0.41 (1)	3.73
P- Q	-4458 / 0	-84.3	-84.3	0.41 (1)	3.73
Q- D	-4458 / 0	-84.3	-84.3	0.41 (1)	3.73
D- E	-3629 / 0	-84.3	-84.3	0.26 (1)	4.27
E- F	-3629 / 0	-84.3	-84.3	0.22 (1)	4.30
F- G	-3629 / 0	-84.3	-84.3	0.22 (1)	4.30
G- H	-2300 / 0	-84.3	-84.3	0.18 (1)	5.21
I- H	-1833 / 0	0.0	0.0	0.27 (1)	7.35
O- B	-2675 / 0	0.0	0.0	0.19 (1)	6.33
O- R	0 / 0	-28.0	-28.0	0.19 (2)	10.00
R- S	0 / 0	-28.0	-28.0	0.19 (2)	10.00
S- N	0 / 0	-28.0	-28.0	0.19 (2)	10.00
N- T	0 / 2686	-28.0	-28.0	0.67 (1)	10.00
T- U	0 / 2686	-28.0	-28.0	0.67 (1)	10.00
U- V	0 / 2686	-28.0	-28.0	0.67 (1)	10.00
V- M	0 / 2686	-28.0	-28.0	0.67 (1)	10.00
M- L	0 / 4458	-28.0	-28.0	0.77 (1)	10.00
L- K	0 / 4458	-28.0	-28.0	0.77 (1)	10.00
K- J	0 / 2300	-28.0	-28.0	0.37 (1)	10.00
J- I	0 / 0	-28.0	-28.0	0.08 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- C	-170 / 231	0.04 (3)
C- M	0 / 2150	0.38 (1)
M- D	0 / 237	0.04 (3)
D- K	-1017 / 0	0.49 (1)
K- E	-375 / 0	0.06 (1)
K- G	0 / 1673	0.30 (1)
J- G	-1482 / 0	0.25 (1)
J- H	0 / 2833	0.50 (1)
B- N	0 / 2753	0.49 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	4-0-11	-23	-25	—	FRONT	VERT	DEAD
C	4-0-11	-222	-222	—	FRONT	VERT	SNOW
P	4-9-4	-113	-113	—	FRONT	VERT	TOTAL
Q	6-9-4	-101	-101	—	FRONT	VERT	TOTAL
R	9-4	-43	-76	—	FRONT	VERT	TOTAL
S	2-9-4	-40	-70	—	FRONT	VERT	TOTAL
T	4-9-4	-40	-70	—	FRONT	VERT	TOTAL
U	6-9-4	-40	-70	—	FRONT	VERT	TOTAL
V	8-8-8	-1157	-1157	—	FRONT	VERT	TOTAL

[M]

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.41/1.00 (C-D:1) , BC=0.77/1.00 (K-M:1) , WB=0.50/1.00 (H-J:1) , SSI=0.74/1.00 (M-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1687	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.65 (L) (INPUT = 1.00)

DWG NO. TAM 26663-8
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43994	DRWG NO.
285813	T8Z4	1	1	TRUSS DESC.	

Temarack Roof Truss, Burlington

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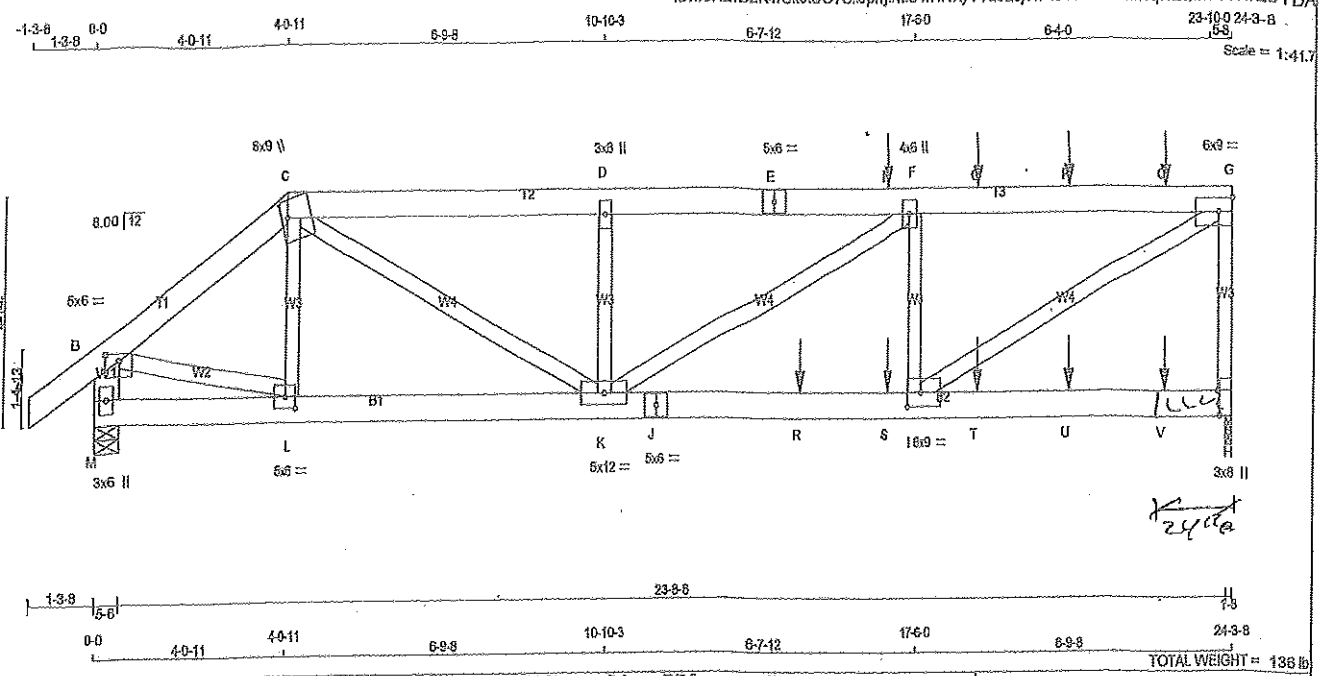


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - G	2x6	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x6	DRY	No.2	SPF
M - J	2x6	DRY	2100F 1.8E	SPF
J - H	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW-p	MT20	5.0	8.0	1.50	3.00
C	TTWW+m	MT20	8.0	9.0	3.75	1.75
D	TMW+w	MT20	3.0	6.0		
E	TS-t	MT20	5.0	6.0		
F	TMWW+t	MT20	4.0	6.0		
G	TMW-t	MT20	6.0	9.0		Edge
H	BMV1+p	MT20	3.0	8.0	5.50	0.50
I	BMWW-t	MT20	6.0	9.0	3.00	3.75
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	5.0	12.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.75
M	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 101.4 lbs FACTORED DOWN AT 16-10-12, 101.4 lbs FACTORED DOWN AT 18-10-12, AND 101.4 lbs FACTORED DOWN AT 20-10-12, AND 101.4 lbs FACTORED DOWN AT 22-10-12 ON TOP CHORD, AND 1340.4 lbs FACTORED DOWN AT 14-11-8, 69.9 lbs FACTORED DOWN AT 16-10-12, 69.9 lbs FACTORED DOWN AT 18-10-12, AND 69.9 lbs FACTORED DOWN AT 20-10-12, AND 69.9 lbs FACTORED DOWN AT 22-10-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

SCAB JOINT # H & ~~I~~ WITH 2 X 6 #2 SPF 24" LONG / SIDE(S) USING 2 ROW(S) 3¼" ARDOX SPIRAL/~~COMMON WIRE~~ NAILS AT 2" O/C STAGGERED (12 NAILS/SCAB). REQUIRED FOR 1/4 SRG PROVISION.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	2652	0	2652	0	0	1-8	4.25
M	2100	0	2100	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	2178	1209 / 0	486 / 0	0 / 0	0 / 0	473 / 0	0 / 0
M	1704	979 / 0	366 / 0	0 / 0	0 / 0	359 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.61 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 33	-84.3	-84.3	0.07 (1)	10.00
B-C	-2365 / 0	-84.3	-84.3	0.16 (1)	5.17
C-D	-4311 / 0	-84.3	-84.3	0.43 (1)	3.79
D-E	-4311 / 0	-84.3	-84.3	0.51 (1)	3.66
E-N	-4311 / 0	-84.3	-84.3	0.51 (1)	3.66
N-F	-4311 / 0	-84.3	-84.3	0.51 (1)	3.66
F-O	-4173 / 0	-84.3	-84.3	0.59 (1)	3.61
O-P	-4173 / 0	-84.3	-84.3	0.59 (1)	3.61
P-Q	-4173 / 0	-84.3	-84.3	0.59 (1)	3.61
Q-G	-4173 / 0	-84.3	-84.3	0.59 (1)	3.61
H-G	-2699 / 0	0.0	0.0	0.60 (1)	5.26
M-B	-2029 / 0	0.0	0.0	0.14 (1)	7.06
M-L	0 / 0	-28.0	-28.0	0.04 (1)	10.00
L-K	0 / 1946	-28.0	-28.0	0.33 (1)	10.00
K-J	0 / 4173	-28.0	-28.0	0.65 (1)	10.00
J-R	0 / 4173	-28.0	-28.0	0.65 (1)	10.00
R-S	0 / 4173	-28.0	-28.0	0.65 (1)	10.00
S-I	0 / 4173	-28.0	-28.0	0.65 (1)	10.00
I-T	0 / 0	-28.0	-28.0	0.31 (1)	10.00
T-U	0 / 0	-28.0	-28.0	0.31 (1)	10.00
U-V	0 / 0	-28.0	-28.0	0.31 (1)	10.00
V-H	0 / 0	-28.0	-28.0	0.31 (1)	10.00

WEBS

MEMB. FR-TO	FORCE (LBS)	MAX CSI (LC)
L-C	-373 / 26	0.05 (1)
C-K	0 / 2678	0.47 (1)
K-D	-571 / 0	0.10 (1)
K-F	0 / 157	0.03 (1)
I-F	-994 / 0	0.17 (1)
I-G	0 / 4736	0.84 (1)
B-L	0 / 2097	0.35 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
N	16-10-12	-101	-101	--	FRONT	VERT	TOTAL
O	18-10-12	-101	-101	--	FRONT	VERT	TOTAL
P	20-10-12	-101	-101	--	FRONT	VERT	TOTAL
Q	22-10-12	-101	-101	--	FRONT	VERT	TOTAL
R	14-11-8	-1340	-1340	--	FRONT	VERT	TOTAL
S	16-10-12	-40	-70	--	FRONT	VERT	TOTAL
T	18-10-12	-40	-70	--	FRONT	VERT	TOTAL
U	20-10-12	-40	-70	--	FRONT	VERT	TOTAL
V	22-10-12	-40	-70	--	FRONT	VERT	TOTAL

DESIGN CRITERIA

[M]

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 063-09
- TPIC 2011

(55 % OF 31.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.27")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 684 (0.43")

CSI: TC=0.60/1.00 (G-H:1) , BC=0.86/1.00 (I-K:1) , WB=0.84/1.00 (G-I:1) , SSI=0.69/1.00 (I-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

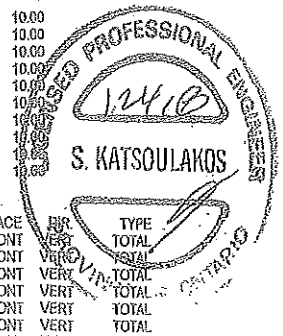
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1658

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.68 (i) (INPUT = 0.90)
JSI METAL= 0.70 (i) (INPUT = 1.00)



DWG NO. TAM5069-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285994	T9	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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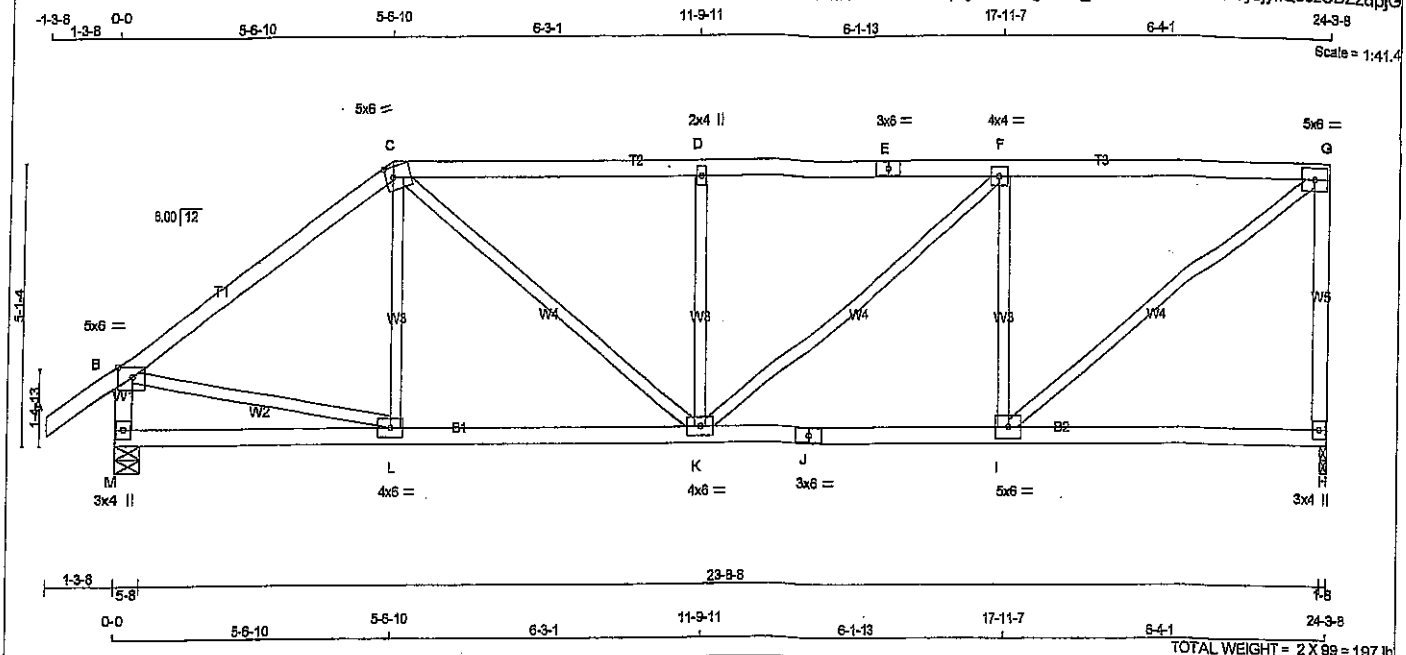

TOTAL WEIGHT = 2 X 99 = 197 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TTWW-m	MT20	5.0	6.0	2.25	1.75
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H	1353	0	1353	0	0	1-8	1-8
M	1479	0	1479	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1119	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
M	1198	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.34 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	FORCE (LBS)	MAX CSI (LC)
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	L-C	-49 / 204	0.05 (3)
B-C	-1493 / 0	-84.3	-84.3	0.55 (1)	4.89	B-L	0 / 1263	0.28 (1)
C-D	-1797 / 0	-84.3	-84.3	0.55 (1)	4.37	I-G	0 / 1778	0.40 (1)
D-E	-1797 / 0	-84.3	-84.3	0.55 (1)	4.34	C-K	0 / 702	0.16 (1)
E-F	-1797 / 0	-84.3	-84.3	0.56 (1)	4.84	I-F	-877 / 0	0.34 (1)
F-G	-1416 / 0	-84.3	-84.3	0.53 (1)	4.81	K-D	-566 / 0	0.22 (1)
H-G	-1292 / 0	0.0	0.0	0.57 (1)	7.10	K-F	0 / 484	0.11 (1)
M-B	-1417 / 0	0.0	0.0	0.15 (1)	6.85			
M-L	0 / 0	-28.0	-28.0	0.24 (3)	10.00			
L-K	0 / 1240	-28.0	-28.0	0.39 (2)	10.00			
K-J	0 / 1416	-28.0	-28.0	0.44 (2)	10.00			
J-I	0 / 1416	-28.0	-28.0	0.44 (2)	10.00			
I-H	0 / 0	-28.0	-28.0	0.27 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.57/1.00 (G-H:1) , BC=0.44/1.00 (I-K:2) , WB=0.40/1.00 (G-I:1) , SSI=0.25/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.86 (K) (INPUT = 0.90)
JSI METAL= 0.44 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
1167
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 3900-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285994	T10	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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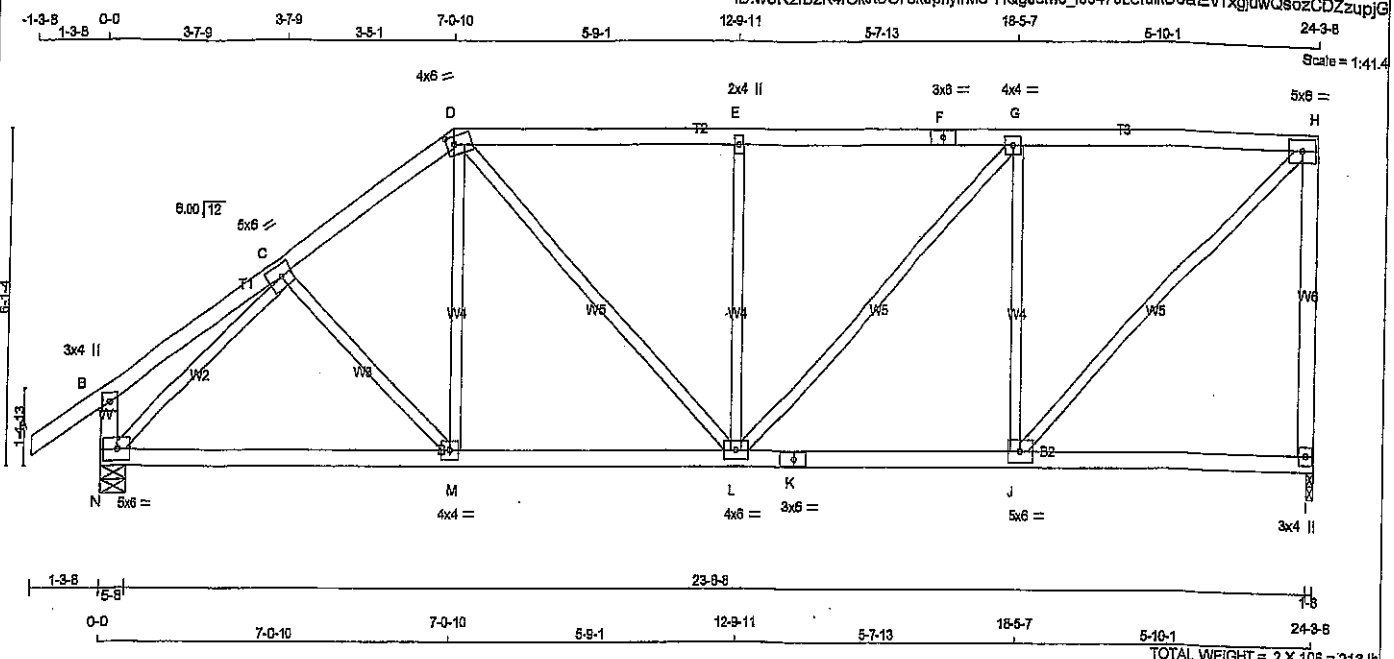

TOTAL WEIGHT = 2 X 106 = 213 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW-m	MT20	4.0	6.0	1.75	1.75
E	TMV+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	5.0		
G	TMWW-t	MT20	4.0	4.0		
H	TMVW-t	MT20	5.0	6.0		
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	5.0	6.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	4.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
I	1363	0	1363	0	0	1-8	1-8
N	1479	0	1479	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1119	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
N	1198	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.86 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 32	-84.3	-84.3	1	0.11 (1)	10.00	C- M	0 / 118	0.03 (3)
B- C	0 / 17	-84.3	-84.3	1	0.15 (1)	10.00	M- D	0 / 307	0.07 (2)
C- D	-1460 / 0	-84.3	-84.3	1	0.18 (1)	5.22	N- C	-1669 / 0	0.64 (1)
D- E	-1473 / 0	-84.3	-84.3	1	0.43 (1)	4.86	J- H	0 / 1557	0.35 (1)
E- F	-1473 / 0	-84.3	-84.3	1	0.44 (1)	4.86	D- L	0 / 385	0.09 (1)
F- G	-1473 / 0	-84.3	-84.3	1	0.44 (1)	4.86	J- G	-812 / 0	0.53 (1)
G- H	-1103 / 0	-84.3	-84.3	1	0.42 (1)	5.46	L- E	-520 / 0	0.30 (1)
I- H	-1298 / 0	0.0	0.0	1	0.94 (1)	7.08	L- G	0 / 530	0.12 (1)
N- B	-233 / 0	0.0	0.0	1	0.02 (1)	7.81			
N- M	0 / 1196	-28.0	-28.0	1	0.45 (2)	10.00			
M- L	0 / 1202	-28.0	-28.0	1	0.47 (2)	10.00			
L- K	0 / 1103	-28.0	-28.0	1	0.37 (2)	10.00			
K- J	0 / 1103	-28.0	-28.0	1	0.37 (2)	10.00			
J- I	0 / 0	-28.0	-28.0	1	0.24 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	45.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.94/1.00 (H-I:1) , BC=0.47/1.00 (L-M:2) , WB=0.64/1.00 (C-N:1) , SSI=0.23/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	622	2284	1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.60 (D) (INPUT = 0.90)
JSI METAL= 0.41 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
11628
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 3989 -18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285994	T11	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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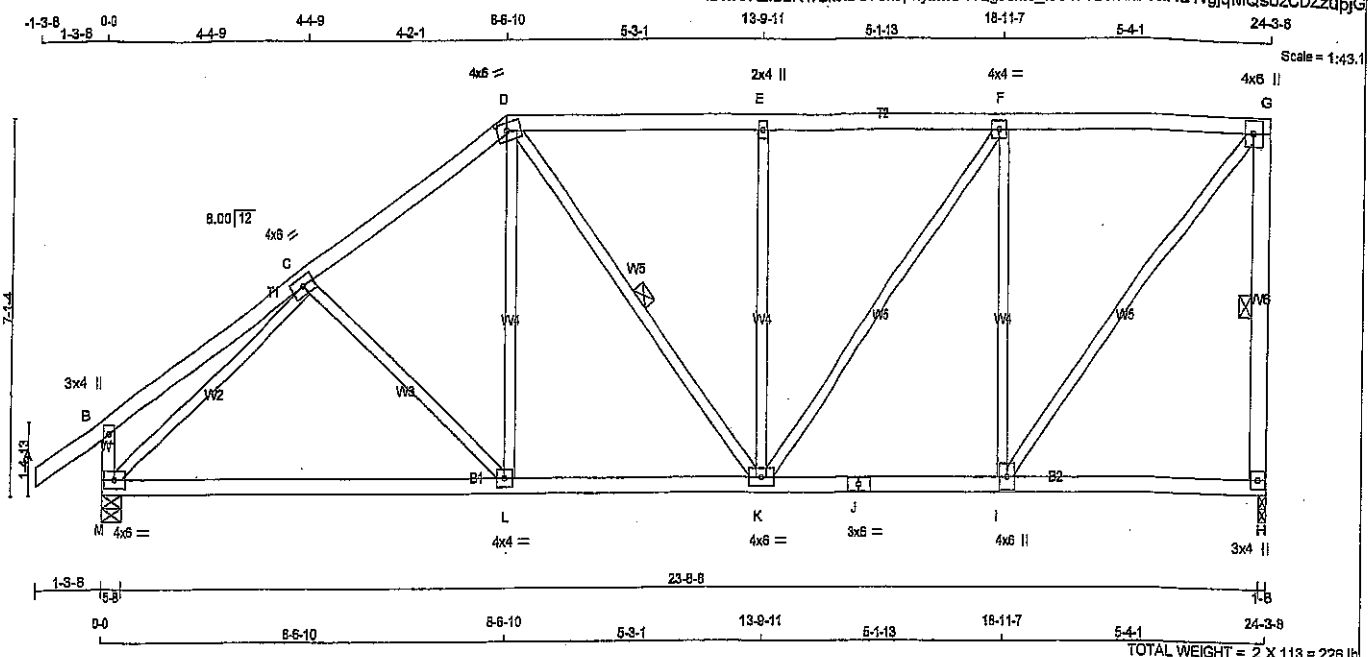

TOTAL WEIGHT = 2 X 113 = 226 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW-m	MT20	4.0	6.0	1.75	2.50
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW+p	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWW+t	MT20	4.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	1363	0	1363	0	0	1-8	1-8
M	1479	0	1479	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LOASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1119	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
M	1198	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0

SEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-H, D-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MEMB.	MAX. UNBRAC LENGTH	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO			
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	C- L	-117 / 65		0.07 (1)
B- C	0 / 23	-84.3	-84.3	0.24 (1)	10.00	L- D	0 / 414		0.09 (2)
C- D	-1390 / 0	-84.3	-84.3	0.28 (1)	5.20	M- C	-1661 / 0		0.93 (1)
D- E	-1226 / 0	-84.3	-84.3	0.34 (1)	5.37	I- G	0 / 1426		0.32 (1)
E- F	-1227 / 0	-84.3	-84.3	0.35 (1)	5.35	D- K	-6 / 141		0.03 (1)
F- G	-579 / 0	-84.3	-84.3	0.34 (1)	6.07	I- F	-948 / 0		0.83 (1)
H- G	-1305 / 0	0.0	0.0	0.30 (1)	5.68	K- E	-474 / 0		0.41 (1)
M- B	-255 / 0	0.0	0.0	0.03 (1)	7.81	K- F	0 / 576		0.13 (1)
M- L	0 / 1223	-28.0	-28.0	0.59 (2)	10.00				
L- K	0 / 1141	-28.0	-28.0	0.59 (2)	10.00				
K- J	0 / 879	-28.0	-28.0	0.31 (2)	10.00				
J- I	0 / 879	-28.0	-28.0	0.31 (2)	10.00				
I- H	0 / 0	-28.0	-28.0	0.21 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.23")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 779 (0.37")

CSI: TC=0.35/1.00 (E-F:1) , BC=0.59/1.00 (K-L:2) , WB=0.93/1.00 (C-M:1) , SSI=0.21/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (M) (INPUT = 0.90)
JSI METAL= 0.40 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
1168
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 3990 -13
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43854	DRWG NO.
285994	T12	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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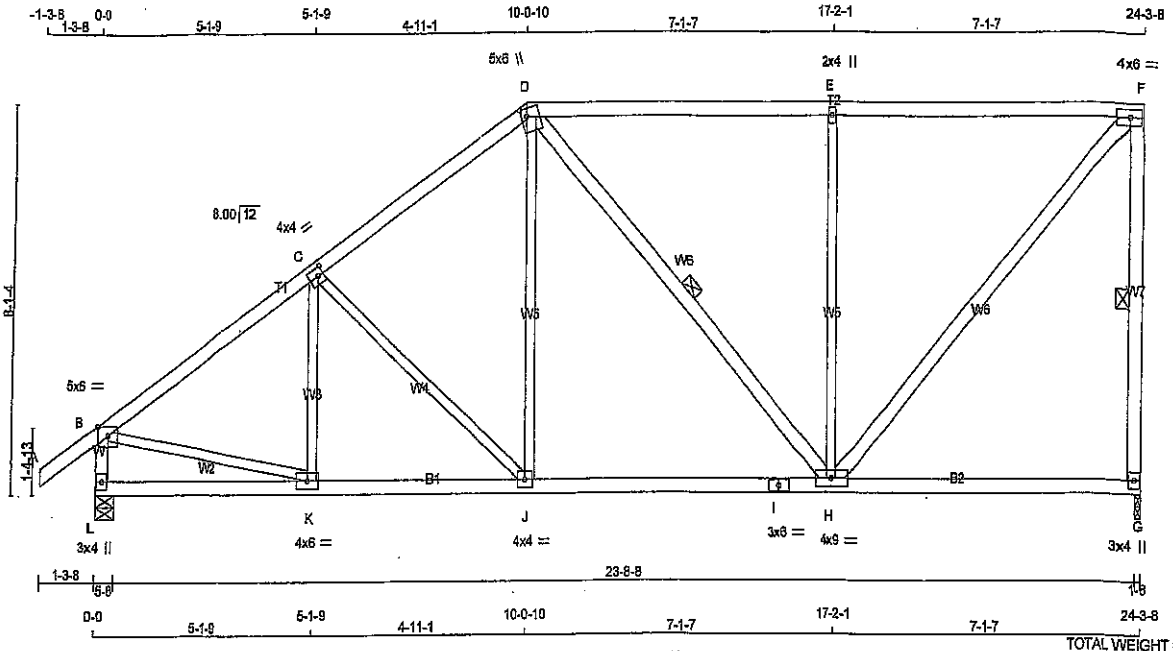


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1383	0	1383	0	0	1-8	1-8
L	1479	0	1479	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1119	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
L	1198	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.81 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

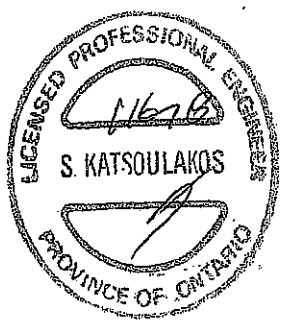
1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	K-C	-123 / 133	0.04 (1)
B-C	-1529 / 0	-84.3	-84.3	0.43 (1)	4.83	C-J	-347 / 0	0.29 (1)
C-D	-1275 / 0	-84.3	-84.3	0.40 (1)	5.20	J-D	0 / 436	0.10 (2)
D-E	-957 / 0	-84.3	-84.3	0.80 (1)	4.81	D-H	-110 / 0	0.05 (1)
E-F	-957 / 0	-84.3	-84.3	0.81 (1)	4.81	H-E	-745 / 0	0.94 (1)
G-F	-1284 / 0	0.0	0.0	0.38 (1)	5.69	H-F	0 / 1432	0.23 (1)
L-B	-1416 / 0	0.0	0.0	0.15 (1)	5.85	B-K	0 / 1323	0.30 (1)
L-K	0 / 0	-28.0	-28.0	0.16 (3)	10.00			
K-J	0 / 1295	-28.0	-28.0	0.33 (2)	10.00			
J-I	0 / 1041	-28.0	-28.0	0.49 (2)	10.00			
I-H	0 / 1041	-28.0	-28.0	0.49 (2)	10.00			
H-G	0 / 0	-28.0	-28.0	0.37 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.81/1.00 (E-F:1) , BC=0.49/1.00 (H-J:2) , WB=0.94/1.00 (E-H:1) , SSI=0.29/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PU) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	822	2284	1858

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL= 0.38 (I) (INPUT = 1.00)

DWG NO. TAM 3991-13
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
286039	T13	5	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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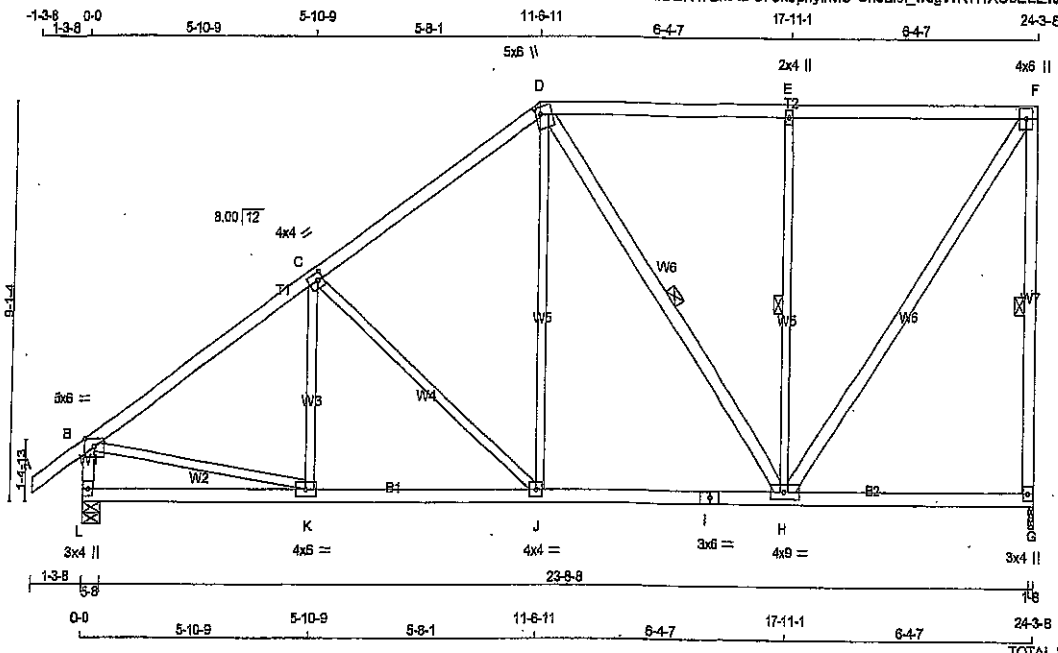


TOTAL WEIGHT = 5 X 123 = 614 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	8.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1363	0	1363	0	0	1-8	1-8
L	1479	0	1479	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1119	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
L	1198	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.61 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

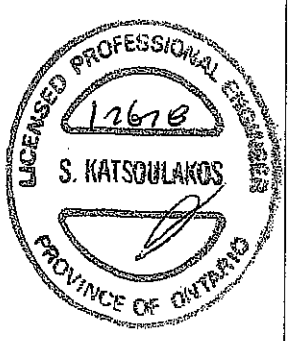
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	K-C	-56 / 203	0.05 (3)
B-C	-1531 / 0	-84.3	-84.3	0.59 (1)	4.61	C-J	-474 / 0	0.56 (1)
C-D	-1167 / 0	-84.3	-84.3	0.54 (1)	5.16	J-D	0 / 490	0.11 (2)
D-E	-790 / 0	-84.3	-84.3	0.51 (1)	5.73	D-H	-270 / 0	0.17 (1)
E-F	-790 / 0	-84.3	-84.3	0.62 (1)	5.73	H-E	-666 / 0	0.36 (1)
G-F	-1292 / 0	0.0	0.0	0.50 (1)	5.68	H-F	0 / 1344	0.22 (1)
L-B	-1412 / 0	0.0	0.0	0.15 (1)	6.86	B-K	0 / 1322	0.30 (1)
L-K	0 / 0	-28.0	-28.0	0.23 (3)	10.00			
K-J	0 / 1300	-28.0	-28.0	0.38 (2)	10.00			
J-I	0 / 948	-28.0	-28.0	0.40 (2)	10.00			
I-H	0 / 948	-28.0	-28.0	0.40 (2)	10.00			
H-G	0 / 0	-28.0	-28.0	0.29 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.62/1.00 (E-F:1) , BC=0.40/1.00 (H-J:2) , WB=0.56/1.00 (C-J:1) , SSI=0.26/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.32 (K) (INPUT = 1.00)

DWG NO. TAM 3993 -18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285813	T14	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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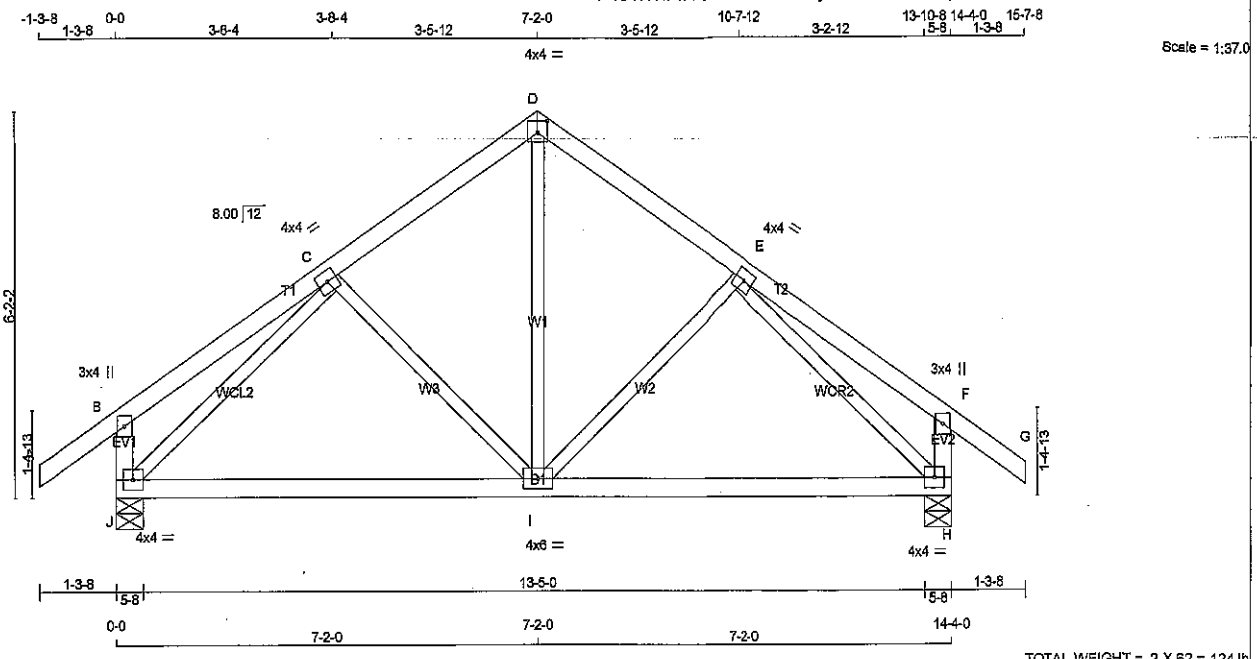

TOTAL WEIGHT = 2 X 62 = 124 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
J	920	0	920	0	0	5-8	5-8
H	920	0	920	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	739	437 / 0	151 / 0	0 / 0	0 / 0	152 / 0	0 / 0
H	739	437 / 0	151 / 0	0 / 0	0 / 0	152 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	I- D	0 / 460	0.10 (1)
B- C	0 / 19	-84.3	-84.3	0.17 (1)	10.00	I- E	-152 / 44	0.06 (1)
C- D	-645 / 0	-84.3	-84.3	0.13 (1)	6.25	C-I	-152 / 44	0.06 (1)
D- E	-645 / 0	-84.3	-84.3	0.13 (1)	6.25	J- C	-879 / 0	0.35 (1)
E- F	0 / 19	-84.3	-84.3	0.17 (1)	10.00	E- H	-879 / 0	0.35 (1)
F- G	0 / 32	-84.3	-84.3	0.11 (1)	10.00			
J- B	-232 / 0	0.0	0.0	0.02 (1)	7.81			
H- F	-232 / 0	0.0	0.0	0.02 (1)	7.81			
J- I	0 / 631	-28.0	-28.0	0.49 (2)	10.00			
I- H	0 / 631	-28.0	-28.0	0.49 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.46")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.46")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.17/1.00 (E-F:1) , BC=0.49/1.00 (H-I:2) , WB=0.35/1.00 (E-H:1) , SSI=0.17/1.00 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (D) (INPUT = 0.90)
JSI METAL= 0.31 (C) (INPUT = 1.00)

DWG NO. TAM 21231-B
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285862	T15A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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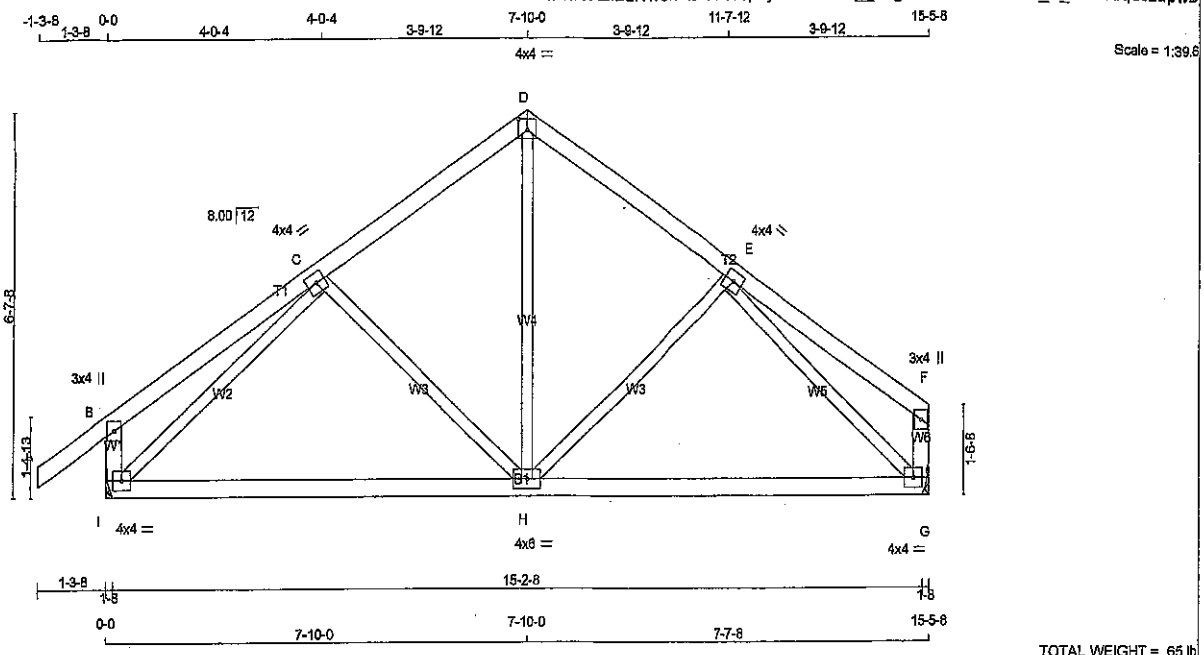


TOTAL WEIGHT = 65 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BMWWW-t	MT20	4.0	6.0		
I	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	983	0	983	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
G	868	0	868	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	791	466 / 0	162 / 0	0 / 0	0 / 0	163 / 0	0 / 0
G	712	395 / 0	162 / 0	0 / 0	0 / 0	155 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	H- D	0 / 494	0.11 (1)	
B- C	0 / 21	-84.3	-84.3	0.20 (1)	10.00	H- E	-148 / 53	0.07 (1)	
C- D	-698 / 0	-84.3	-84.3	0.16 (1)	6.25	C- H	-182 / 41	0.09 (1)	
D- E	-697 / 0	-84.3	-84.3	0.15 (1)	6.25	I- C	-957 / 0	0.45 (1)	
E- F	0 / 21	-84.3	-84.3	0.19 (1)	10.00	E- G	-948 / 0	0.42 (1)	
I- B	-242 / 0	0.0	0.0	0.02 (1)	7.81				
G- F	-118 / 0	0.0	0.0	0.01 (1)	7.81				
I- H	0 / 696	-28.0	-28.0	0.56 (2)	10.00				
H- G	0 / 672	-28.0	-28.0	0.56 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.52")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.52")
CALCULATED VERT. DEFL.(TL) = L/ 981 (0.19")

CSI: TC=0.20/1.00 (B-C:1) , BC=0.56/1.00 (H-I:2) , WB=0.45/1.00 (C-I:1) , SSI=0.19/1.00 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

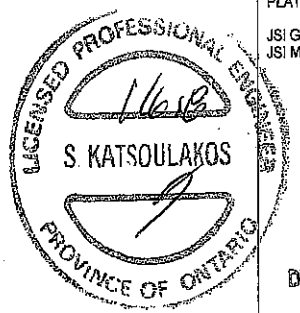
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1856

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (D) (INPUT = 0.90)
JSI METAL= 0.34 (C) (INPUT = 1.00)



DWG NO. TAM4010-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285812	G15A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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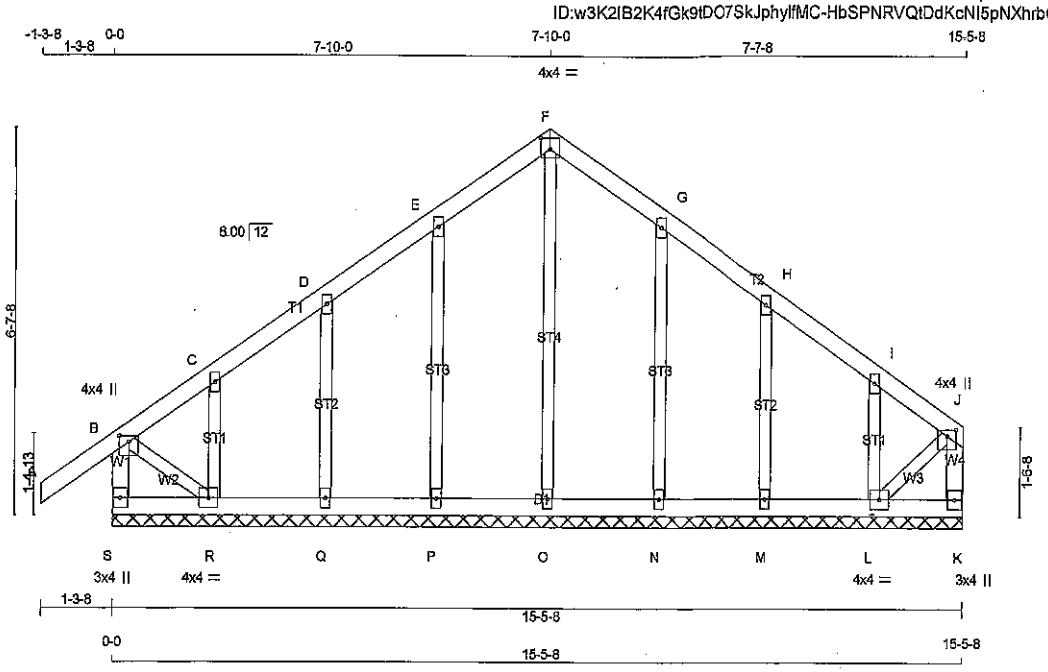

TOTAL WEIGHT = 68 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
S - B	2x4	DRY	No.2	SPF
A - F	2x4	DRY	No.2	SPF
F - J	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
S - K	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C, D, E, G, H, I						
C	TMW+w	MT20	2.0	4.0		
F	TTW-p	MT20	4.0	4.0	2.25	2.00
J	TMVW+p	MT20	4.0	4.0	1.25	2.00
K	BMV1+p	MT20	3.0	4.0		
L	BMWW1-t	MT20	4.0	4.0		
M, N, O, P, Q						
M	BMW1+w	MT20	2.0	4.0		
R	BMWW1-t	MT20	4.0	4.0		
S	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD FROM	TO	LC1 MAX (PLF) CSI (LC)	MAX. UNBRAC LENGTH
S- B	-258 / 0	0.0	0.0	0.03 (1)	7.81
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00
B- C	-50 / 0	-84.3	-84.3	0.11 (1)	6.25
C- D	-19 / 0	-84.3	-84.3	0.04 (1)	6.25
D- E	-18 / 0	-84.3	-84.3	0.05 (1)	6.25
E- F	-25 / 0	-84.3	-84.3	0.05 (1)	6.25
F- G	-25 / 0	-84.3	-84.3	0.05 (1)	6.25
G- H	-17 / 0	-84.3	-84.3	0.05 (1)	6.25
H- I	-24 / 0	-84.3	-84.3	0.04 (1)	6.25
I- J	-21 / 0	-84.3	-84.3	0.04 (1)	6.25
K- J	-84 / 0	0.0	0.0	0.01 (1)	7.81
S- R	0 / 0	-28.0	-28.0	0.03 (2)	10.00
R- Q	0 / 20	-28.0	-28.0	0.03 (2)	10.00
Q- P	0 / 16	-28.0	-28.0	0.02 (2)	10.00
P- O	0 / 13	-28.0	-28.0	0.02 (2)	10.00
O- N	0 / 13	-28.0	-28.0	0.02 (2)	10.00
N- M	0 / 16	-28.0	-28.0	0.02 (2)	10.00
M- L	0 / 20	-28.0	-28.0	0.02 (2)	10.00
L- K	0 / 0	-28.0	-28.0	0.02 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- F	-124 / 0	0.09 (1)
P- E	-189 / 0	0.08 (1)
Q- D	-172 / 0	0.04 (1)
R- C	-108 / 0	0.02 (1)
N- G	-191 / 0	0.08 (1)
M- H	-162 / 0	0.04 (1)
L- I	-162 / 0	0.03 (1)
B- R	0 / 28	0.01 (1)
L- J	0 / 32	0.01 (1)

DWG NO. TAM5002G17

STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11 (A-B:1) , BC=0.03 (Q-R:2) , WB=0.09 (F-O:1) , SSI=0.08 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (B) (INPUT = 0.90)
JSI METAL= 0.05 (G) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
286039	T37	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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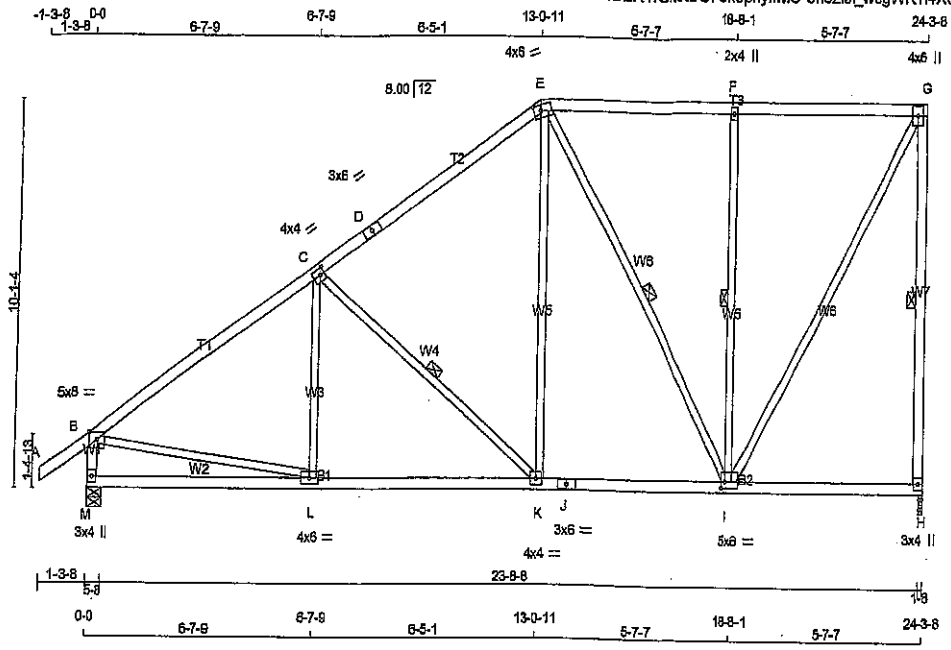

Scale = 1:59.4

TOTAL WEIGHT = 2 X 122 = 243 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	4.0	6.0	1.75	2.50
F	TMW+w	MT20	2.0	4.0		
G	TMVW+p	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWWW-t	MT20	5.0	6.0	2.00	1.50
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1363	0	1363	0	0	1-8	1-8
M	1479	0	1479	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1119	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
M	1198	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.69 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-H, C-K, E-I, F-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	
B-C	-1522 / 0	-84.3	-84.3	0.55 (1)	4.69	
C-D	-1051 / 0	-84.3	-84.3	0.49 (1)	5.48	
D-E	-1051 / 0	-84.3	-84.3	0.49 (1)	5.48	
E-F	-644 / 0	-84.3	-84.3	0.34 (1)	6.25	
F-G	-644 / 0	-84.3	-84.3	0.34 (1)	6.25	
H-G	-1299 / 0	0.0	0.0	0.65 (1)	5.68	
M-B	-1404 / 0	0.0	0.0	0.14 (1)	6.87	
M-L	0 / 0	-28.0	-28.0	0.31 (3)	10.00	
L-K	0 / 1296	-28.0	-28.0	0.46 (2)	10.00	
K-J	0 / 848	-28.0	-28.0	0.31 (2)	10.00	
J-I	0 / 848	-28.0	-28.0	0.31 (2)	10.00	
I-H	0 / 0	-28.0	-28.0	0.21 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FORCE (LBS)	MAX CSI (LC)
L-C	-1 / 267	0.06 (3)
C-K	-594 / 0	0.30 (1)
K-E	0 / 545	0.12 (2)
E-I	-413 / 0	0.40 (1)
I-F	-586 / 0	0.41 (1)
I-G	0 / 1289	0.28 (1)
B-L	0 / 1314	0.30 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.65/1.00 (G-H:1) , BC=0.46/1.00 (K-L:2) , WB=0.41/1.00 (F-I:1) , SSI=0.23/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

SSI GRIP= 0.83 (L) (INPUT = 0.90)
SSI METAL= 0.32 (L) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
1678
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM3994-13
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285815	T41	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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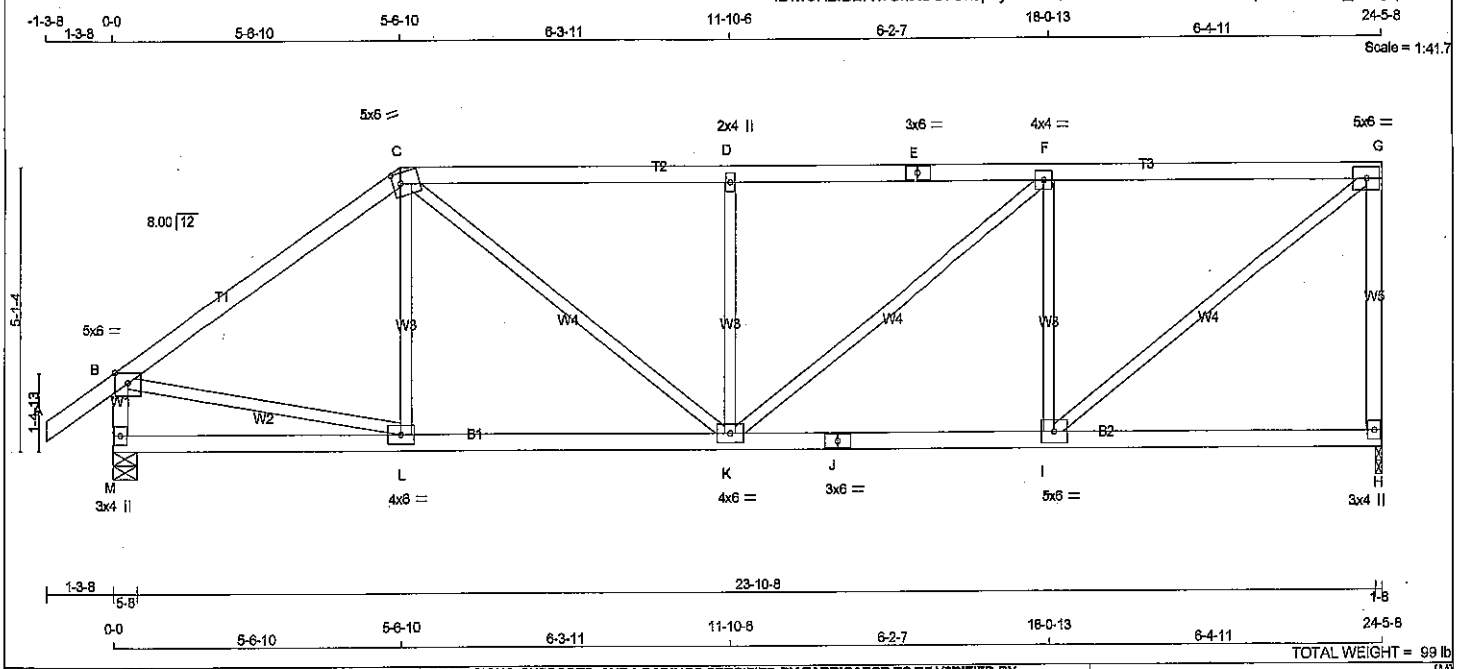

TOTAL WEIGHT = 99 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TTWW-m	MT20	5.0	6.0	2.25	1.75
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1373	0	1373	0	0	1-8	1-8
M	1488	0	1488	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1127	626 / 0	257 / 0	0 / 0	0 / 0	245 / 0	0 / 0
M	1206	698 / 0	257 / 0	0 / 0	0 / 0	253 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.30 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	L- C	-50 / 204	0.05 (3)
B- C	-1507 / 0	-84.3	-84.3	0.55 (1)	4.66	B- L	0 / 1275	0.29 (1)
C- D	-1822 / 0	-84.3	-84.3	0.56 (1)	4.33	I- G	0 / 1799	0.40 (1)
D- E	-1822 / 0	-84.3	-84.3	0.58 (1)	4.30	C- K	0 / 717	0.16 (1)
E- F	-1822 / 0	-84.3	-84.3	0.58 (1)	4.30	I- F	-882 / 0	0.34 (1)
F- G	-1437 / 0	-84.3	-84.3	0.54 (1)	4.77	K- D	-571 / 0	0.22 (1)
H- G	-1300 / 0	0.0	0.0	0.57 (1)	7.08	K- F	0 / 487	0.11 (1)
M- B	-1427 / 0	0.0	0.0	0.15 (1)	6.84			
M- L	0 / 0	-28.0	-28.0	0.24 (3)	10.00			
L- K	0 / 1251	-28.0	-28.0	0.39 (2)	10.00			
K- J	0 / 1437	-28.0	-28.0	0.44 (2)	10.00			
J- I	0 / 1437	-28.0	-28.0	0.44 (2)	10.00			
I- H	0 / 0	-28.0	-28.0	0.28 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.62")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.62")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.58/1.00 (D-F:1) , BC=0.44/1.00 (I-K:2) , WB=0.40/1.00 (G-I:1) , SSI=0.25/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL= 0.44 (J) (INPUT = 1.00)

DWG NO. TAM3363_18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285815	T41A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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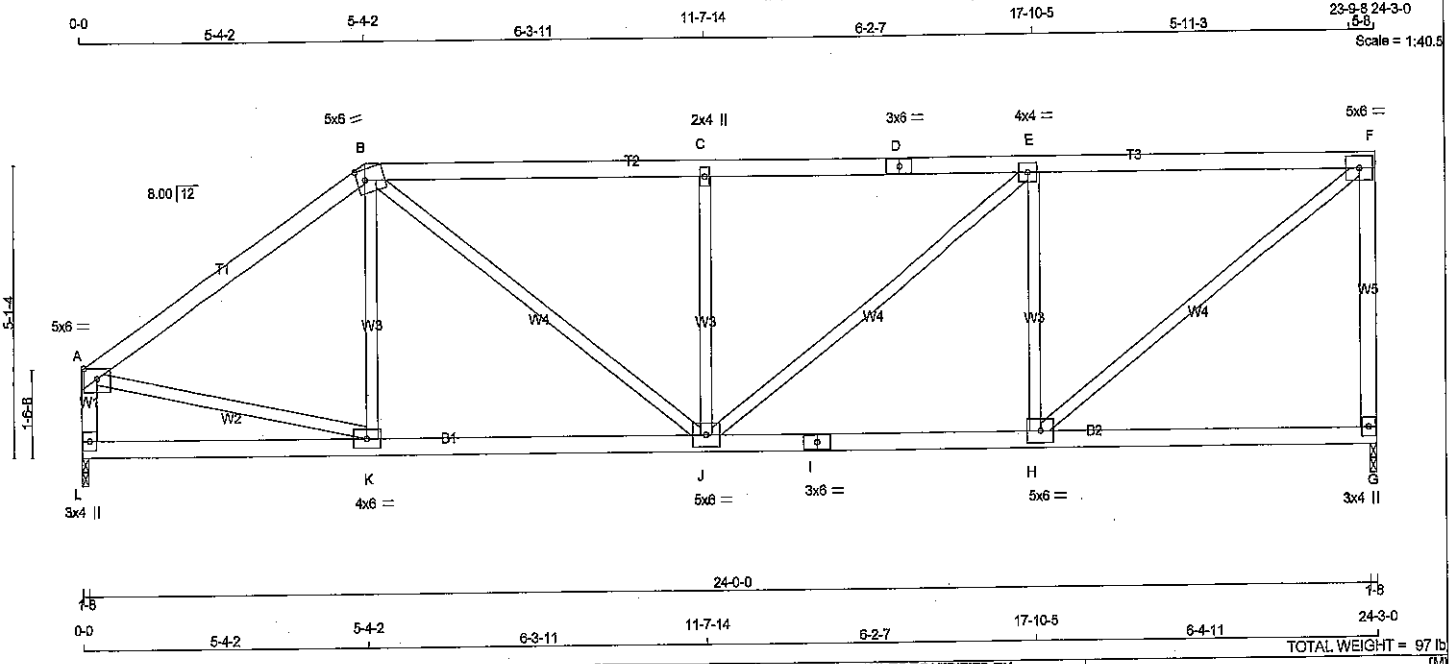

TOTAL WEIGHT = 97 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - A	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	6.0	Edge	
B	TTWW-m	MT20	5.0	6.0	2.25	1.75
C	TMW+w	MT20	2.0	4.0		
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMVW-t	MT20	5.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	5.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	5.0	6.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1361	0	1361	0	0	1-8	1-8
L	1361	0	1361	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1118	620 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
L	1118	620 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.33 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1451 / 0	-84.3	-84.3	0.50 (1)	4.82
B- C	-1791 / 0	-84.3	-84.3	0.56 (1)	4.36
C- D	-1792 / 0	-84.3	-84.3	0.57 (1)	4.33
D- E	-1792 / 0	-84.3	-84.3	0.57 (1)	4.33
E- F	-1422 / 0	-84.3	-84.3	0.54 (1)	4.79
G- F	-1289 / 0	0.0	0.0	0.57 (1)	7.10
L- A	-1302 / 0	0.0	0.0	0.14 (1)	7.06
L- K	0 / 0	-28.0	-28.0	0.23 (3)	10.00
K- J	0 / 1204	-28.0	-28.0	0.36 (2)	10.00
J- I	0 / 1422	-28.0	-28.0	0.44 (2)	10.00
I- H	0 / 1422	-28.0	-28.0	0.44 (2)	10.00
H- G	0 / 0	-28.0	-28.0	0.28 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- B	-85 / 181	0.04 (3)
A- K	0 / 1236	0.28 (1)
H- F	0 / 1780	0.40 (1)
B- J	0 / 738	0.17 (1)
H- E	-870 / 0	0.33 (1)
J- C	-571 / 0	0.22 (1)
J- E	0 / 488	0.11 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.57/1.00 (C-E:1) , BC=0.44/1.00 (H-J:2) , WB=0.40/1.00 (F-H:1) , SSI=0.25/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.65 (K) (INPUT = 0.90)
JSI METAL= 0.44 (I) (INPUT = 1.00)

DWG NO. TAM 5072-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285815	T42	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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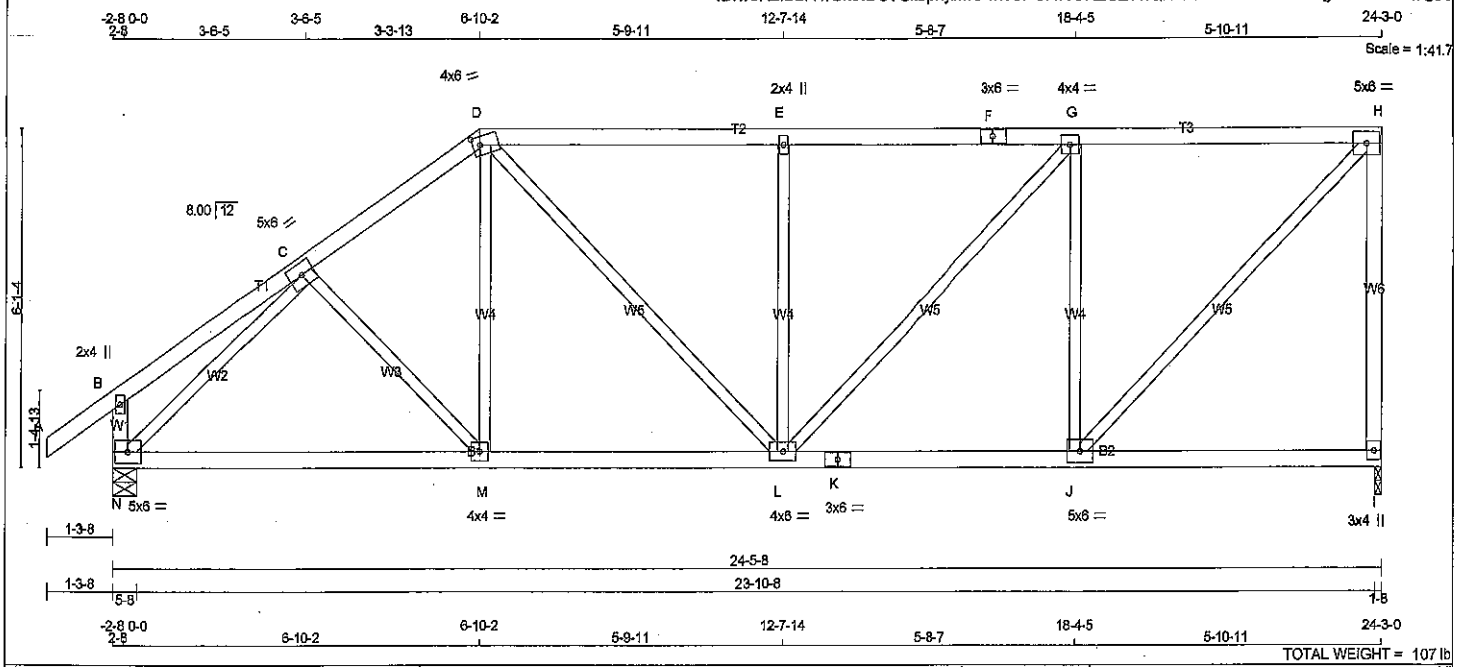

-2-8 0-0 | 2-8 | 3-6-5 | 3-6-5 | 3-3-13 | 6-10-2 | 5-9-11 | 12-7-14 | 5-8-7 | 18-4-5 | 5-10-11 | 24-3-0

-2-8 0-0 | 2-8 | 6-10-2 | 6-10-2 | 5-9-11 | 12-7-14 | 5-8-7 | 18-4-5 | 5-10-11 | 24-3-0

TOTAL WEIGHT = 107 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW+w	MT20	2.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW-m	MT20	4.0	6.0	1.75	1.75
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TMWW-t	MT20	5.0	6.0		
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	5.0	6.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	4.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMWW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	1364	0	1364	0	0	1-8	1-8
N	1493	0	1493	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1120	622 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
N	1208	700 / 0	255 / 0	0 / 0	0 / 0	252 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.84 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
FR-TO						
A- B	0 / 34	-84.3	-84.3	0.12 (1)	10.00	
B- C	0 / 37	-84.3	-84.3	0.15 (1)	10.00	
C- D	-1442 / 0	-84.3	-84.3	0.18 (1)	5.26	
D- E	-1477 / 0	-84.3	-84.3	0.44 (1)	4.86	
E- F	-1477 / 0	-84.3	-84.3	0.45 (1)	4.84	
F- G	-1477 / 0	-84.3	-84.3	0.45 (1)	4.84	
G- H	-1112 / 0	-84.3	-84.3	0.43 (1)	5.43	
I- H	-1298 / 0	0.0	0.0	0.94 (1)	7.08	
N- M	0 / 1161	-28.0	-28.0	0.44 (2)	10.00	
M- L	0 / 1186	-28.0	-28.0	0.45 (2)	10.00	
L- K	0 / 1112	-28.0	-28.0	0.37 (2)	10.00	
K- J	0 / 1112	-28.0	-28.0	0.37 (2)	10.00	
J- I	0 / 0	-28.0	-28.0	0.24 (3)	10.00	

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
C- M	0 / 128	0.03 (3)
M- D	0 / 288	0.06 (2)
J- H	0 / 1582	0.35 (1)
D- L	0 / 410	0.09 (1)
J- G	-909 / 0	0.53 (1)
L- E	-525 / 0	0.31 (1)
L- G	0 / 521	0.12 (1)
N- B	-230 / 0	0.02 (1)
N- C	-1689 / 0	0.68 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF . PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.94/1.00 (H-I:1) , BC=0.45/1.00 (L-M:2) , WB=0.88/1.00 (C-N:1) , SSI=0.23/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (D) (INPUT = 0.90)
JSI METAL= 0.41 (C) (INPUT = 1.00)

DWG NO. TAM3364-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285815	T42A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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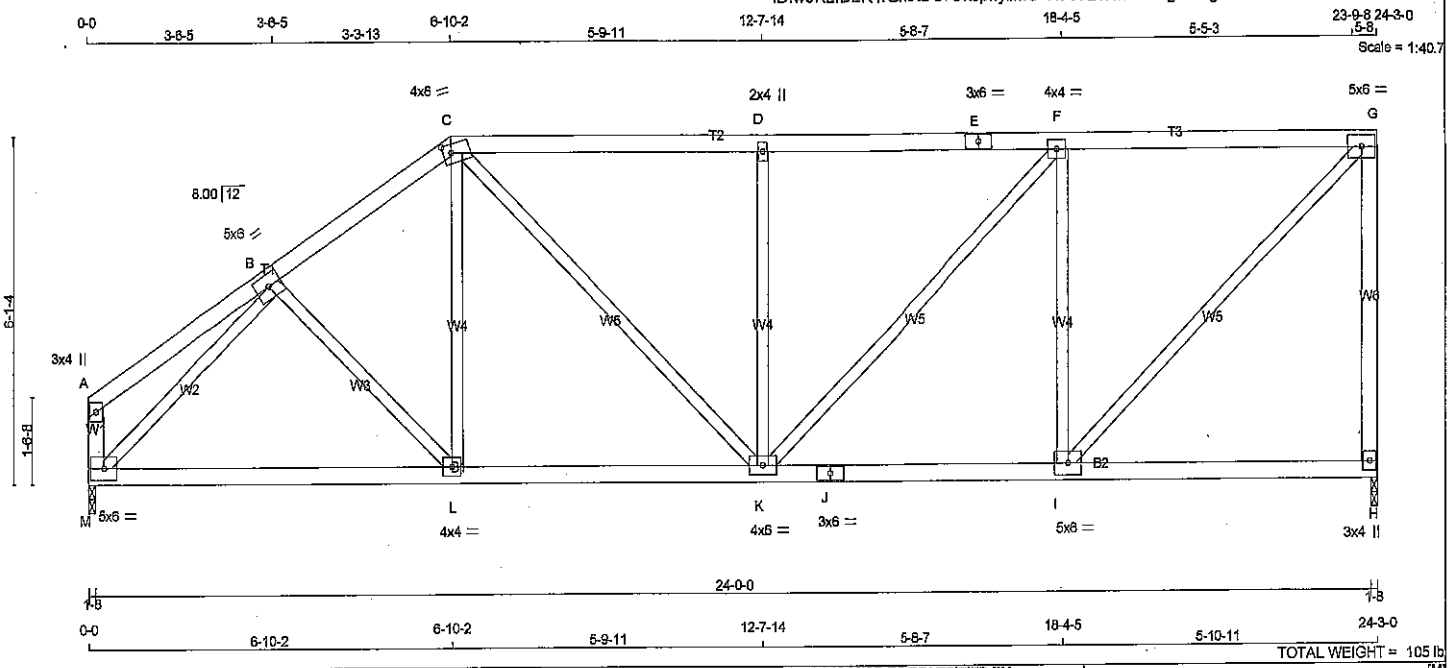

TOTAL WEIGHT = 105 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - A	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	5.0	6.0		
C	TTWW-m	MT20	4.0	6.0	1.75	1.75
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1361	0	1361	0	0	1-8	1-8
M	1361	0	1361	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1118	620 / 0	255 / 0	0 / 0	0 / 0	242 / 0	0 / 0
M	1118	620 / 0	255 / 0	0 / 0	0 / 0	242 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.65 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 17	-84.3	-84.3	0.14 (1)	10.00	B- L	0 / 134	0.03 (3)
B- C	-1430 / 0	-84.3	-84.3	0.17 (1)	5.28	L- C	0 / 279	0.06 (2)
C- D	-1471 / 0	-84.3	-84.3	0.44 (1)	4.88	M- B	-1633 / 0	0.62 (1)
D- E	-1471 / 0	-84.3	-84.3	0.45 (1)	4.85	I- G	0 / 1558	0.35 (1)
E- F	-1471 / 0	-84.3	-84.3	0.45 (1)	4.85	C- K	0 / 415	0.09 (1)
F- G	-1109 / 0	-84.3	-84.3	0.43 (1)	5.43	I- F	-906 / 0	0.53 (1)
H- G	-1295 / 0	0.0	0.0	0.93 (1)	7.10	K- D	-525 / 0	0.31 (1)
M- A	-114 / 0	0.0	0.0	0.01 (1)	7.81	K- F	0 / 517	0.12 (1)
M- L	0 / 1141	-28.0	-28.0	0.43 (2)	10.00			
L- K	0 / 1177	-28.0	-28.0	0.45 (2)	10.00			
K- J	0 / 1109	-28.0	-28.0	0.37 (2)	10.00			
J- I	0 / 1109	-28.0	-28.0	0.37 (2)	10.00			
I- H	0 / 0	-28.0	-28.0	0.24 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.93/1.00 (G-H:1) , BC=0.45/1.00 (K-L:2) , WB=0.62/1.00 (B-M:1) , SSI=0.23/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.66 (C) (INPUT = 0.90)
JSI METAL= 0.40 (B) (INPUT = 1.00)

DWG NO. TAM 5073 -18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285815	T43	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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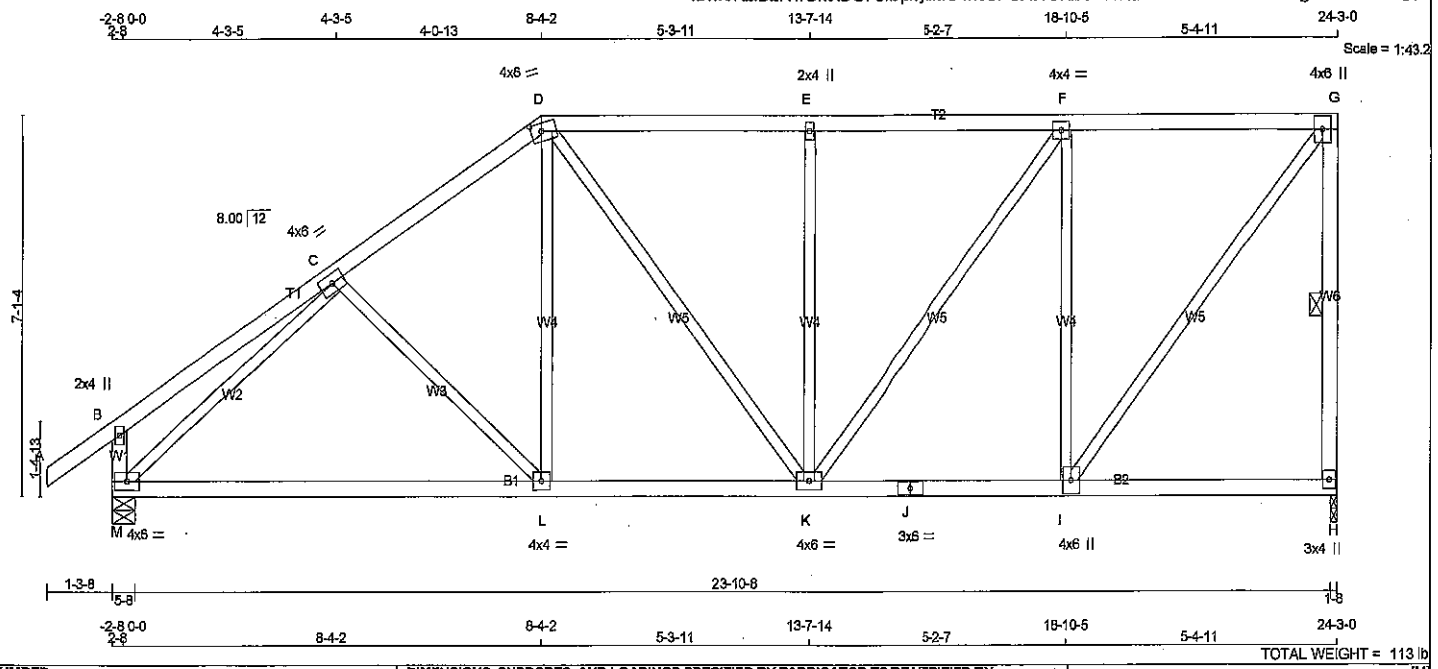


TOTAL WEIGHT = 113 lb

LUMBER 1-3-8

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW+w	MT20	2.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW-m	MT20	4.0	6.0	1.75	2.50
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW+p	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWW+t	MT20	4.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMWW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H	1364	0	1364	0	0	1-6	1-8
M	1493	0	1493	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1120	622 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
M	1208	700 / 0	255 / 0	0 / 0	0 / 0	252 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.22 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-H.

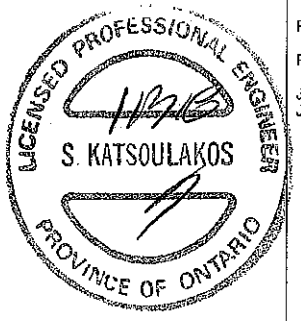
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 34	-84.3	-84.3	0.12 (1)	10.00	
B- C	0 / 44	-84.3	-84.3	0.24 (1)	10.00	
C- D	-1379 / 0	-84.3	-84.3	0.28 (1)	5.22	
D- E	-1232 / 0	-84.3	-84.3	0.35 (1)	5.35	
E- F	-1232 / 0	-84.3	-84.3	0.36 (1)	5.32	
F- G	-887 / 0	-84.3	-84.3	0.34 (1)	6.04	
H- G	-1305 / 0	0.0	0.0	0.30 (1)	5.66	
M- L	0 / 1193	-26.0	-28.0	0.57 (2)	10.00	
L- K	0 / 1131	-26.0	-28.0	0.58 (2)	10.00	
K- J	0 / 887	-26.0	-28.0	0.32 (2)	10.00	
J- I	0 / 887	-26.0	-28.0	0.32 (2)	10.00	
I- H	0 / 0	-26.0	-28.0	0.21 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- L	-90 / 95	0.05 (1)
L- D	0 / 395	0.09 (2)
I- G	0 / 1429	0.32 (1)
D- K	0 / 163	0.04 (1)
I- F	-943 / 0	0.82 (1)
K- E	-479 / 0	0.42 (1)
K- F	0 / 568	0.13 (1)
M- B	-251 / 0	0.03 (1)
M- C	-1666 / 0	0.98 (1)



[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.21")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 831 (0.35")

CSI: TC=0.58/1.00 (E-F:1) , BC=0.58/1.00 (K-L:2) , WB=0.98/1.00 (C-M:1) , SSI=0.21/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (M) (INPUT = 0.90)
JSI METAL= 0.41 (C) (INPUT = 1.00)

DWG NO. TAW3365-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285815	T43A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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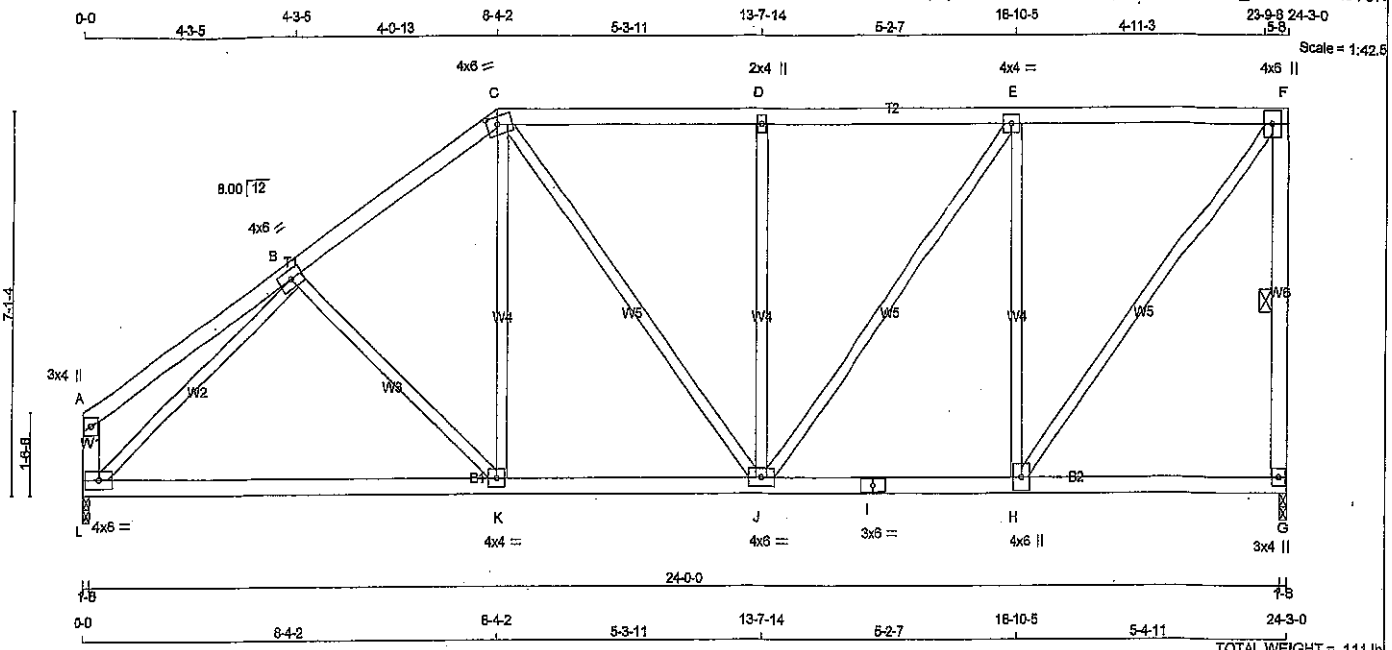

TOTAL WEIGHT = 111 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - A	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	6.0		
C	TTWW-m	MT20	4.0	6.0	1.75	2.50
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMWW+p	MT20	4.0	8.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWW+t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW+t	MT20	4.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1361	0	1361	0	0	1-8	1-8
L	1361	0	1361	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1118	620 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
L	1118	620 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 22	-84.3	-84.3	0.23 (1)	10.00	
B- C	-1369 / 0	-84.3	-84.3	0.27 (1)	5.25	
C- D	-1227 / 0	-84.3	-84.3	0.35 (1)	5.36	
D- E	-1227 / 0	-84.3	-84.3	0.36 (1)	5.33	
E- F	-884 / 0	-84.3	-84.3	0.34 (1)	6.05	
G- F	-1302 / 0	0.0	0.0	0.30 (1)	5.86	
L- A	-136 / 0	0.0	0.0	0.01 (1)	7.81	
L- K	0 / 1178	-28.0	-28.0	0.56 (2)	10.00	
K- J	0 / 1124	-28.0	-28.0	0.57 (2)	10.00	
J- I	0 / 884	-28.0	-28.0	0.32 (2)	10.00	
I- H	0 / 884	-28.0	-28.0	0.32 (2)	10.00	
H- G	0 / 0	-28.0	-28.0	0.21 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- K	-77 / 99	0.05 (1)
K- C	0 / 387	0.09 (2)
L- B	-1629 / 0	0.90 (1)
H- F	0 / 1425	0.32 (1)
C- J	0 / 188	0.04 (1)
H- E	-940 / 0	0.82 (1)
J- D	-479 / 0	0.42 (1)
J- E	0 / 585	0.13 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.21")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 853 (0.34")

CSI: TC=0.36/1.00 (D-E:1) , BC=0.57/1.00 (J-K:2) , WB=0.90/1.00 (B-L:1) , SSI=0.21/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.40 (B) (INPUT = 1.00)

DWG NO. TAM 5074 -18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285815	T44	1	1	TRUSS DESC.	

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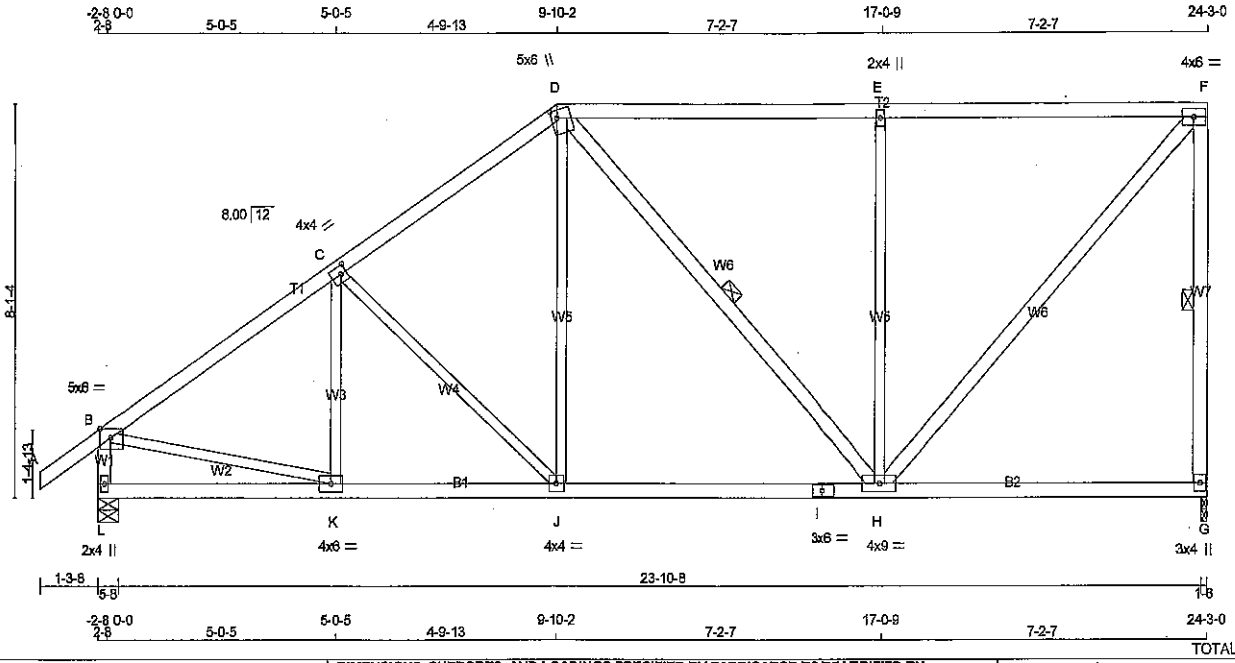


TOTAL WEIGHT = 118 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMW1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1365	0	1365	0	0	1-8	1-8
L	1493	0	1493	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1120	622 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
L	1207	700 / 0	255 / 0	0 / 0	0 / 0	252 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.72 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

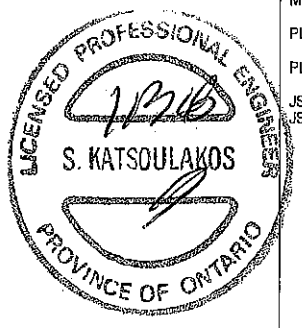
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A-B	0 / 35	-84.3	-84.3	0.14 (1)	10.00	K-C	-147 / 117	0.05 (1)
B-C	-1496 / 0	-84.3	-84.3	0.41 (1)	4.89	C-J	-319 / 0	0.27 (1)
C-D	-1272 / 0	-84.3	-84.3	0.39 (1)	5.22	J-D	0 / 425	0.10 (2)
D-E	-976 / 0	-84.3	-84.3	0.83 (1)	4.72	D-H	-91 / 0	0.05 (1)
E-F	-976 / 0	-84.3	-84.3	0.83 (1)	4.72	H-E	-754 / 0	0.95 (1)
G-F	-1284 / 0	0.0	0.0	0.38 (1)	5.69	H-F	0 / 1436	0.23 (1)
						L-B	-1432 / 0	0.15 (1)
L-K	0 / 0	-28.0	-28.0	0.15 (3)	10.00	B-K	0 / 1301	0.29 (1)
K-J	0 / 1257	-28.0	-28.0	0.33 (2)	10.00			
J-I	0 / 1038	-28.0	-28.0	0.50 (2)	10.00			
I-H	0 / 1038	-28.0	-28.0	0.50 (2)	10.00			
H-G	0 / 0	-28.0	-28.0	0.38 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.83/1.00 (E-F:1) , BC=0.50/1.00 (H-J:2) , WB=0.95/1.00 (E-H:1) , SSI=0.30/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(ORY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (F) (INPUT = 0.90)
JSI METAL= 0.35 (I) (INPUT = 1.00)

DWG NO. TAM 3366-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285815	T44A	1	1	TRUSS DESC.	

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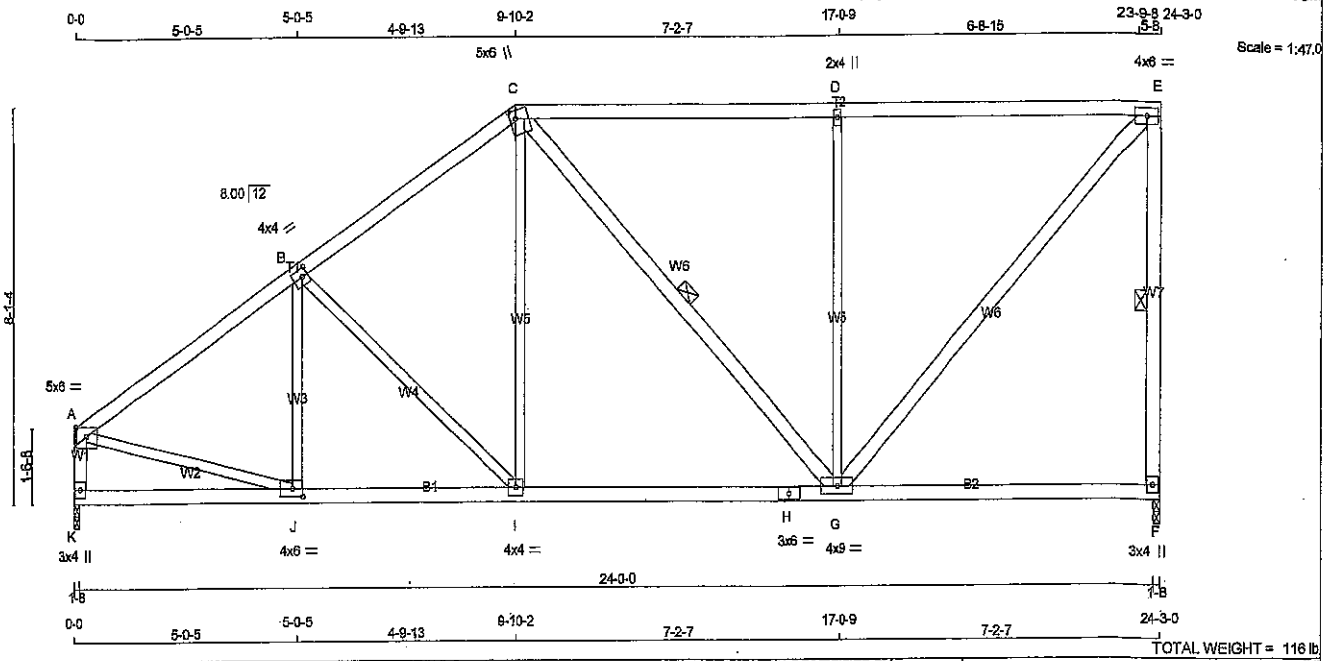

TOTAL WEIGHT = 116 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	6.0	Edge	
B	TMWW-t	MT20	4.0	4.0	2.00	1.50
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW+t	MT20	4.0	9.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMWW-t	MT20	4.0	6.0	2.00	2.75
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1361	0	1361	0	0	1-8	1-8
K	1361	0	1361	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1118	620 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
K	1118	620 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.74 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-F, C-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1477 / 0	-84.3	-84.3	0.40 (1)	4.93
B- C	-1264 / 0	-84.3	-84.3	0.38 (1)	5.24
C- D	-973 / 0	-84.3	-84.3	0.83 (1)	4.74
D- E	-973 / 0	-84.3	-84.3	0.83 (1)	4.74
F- E	-1281 / 0	0.0	0.0	0.38 (1)	5.70
K- A	-1301 / 0	0.0	0.0	0.14 (1)	7.06
K- J	0 / 0	-28.0	-28.0	0.15 (3)	10.00
J- I	0 / 1252	-28.0	-28.0	0.33 (2)	10.00
I- H	0 / 1032	-28.0	-28.0	0.50 (2)	10.00
H- G	0 / 1032	-28.0	-28.0	0.50 (2)	10.00
G- F	0 / 0	-28.0	-28.0	0.38 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- B	-158 / 110	0.06 (1)
B- I	-307 / 0	0.26 (1)
I- C	0 / 419	0.09 (2)
C- G	-67 / 0	0.05 (1)
G- D	-754 / 0	0.95 (1)
G- E	0 / 1431	0.23 (1)
A- J	0 / 1288	0.29 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.83/1.00 (D-E:1) , BC=0.50/1.00 (G-I:2) , WB=0.95/1.00 (D-G:1) , SSI=0.30/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (E) (INPUT = 0.90)
JSI METAL= 0.41 (H) (INPUT = 1.00)

DWG NO. TAM 5075 -18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285815	T45	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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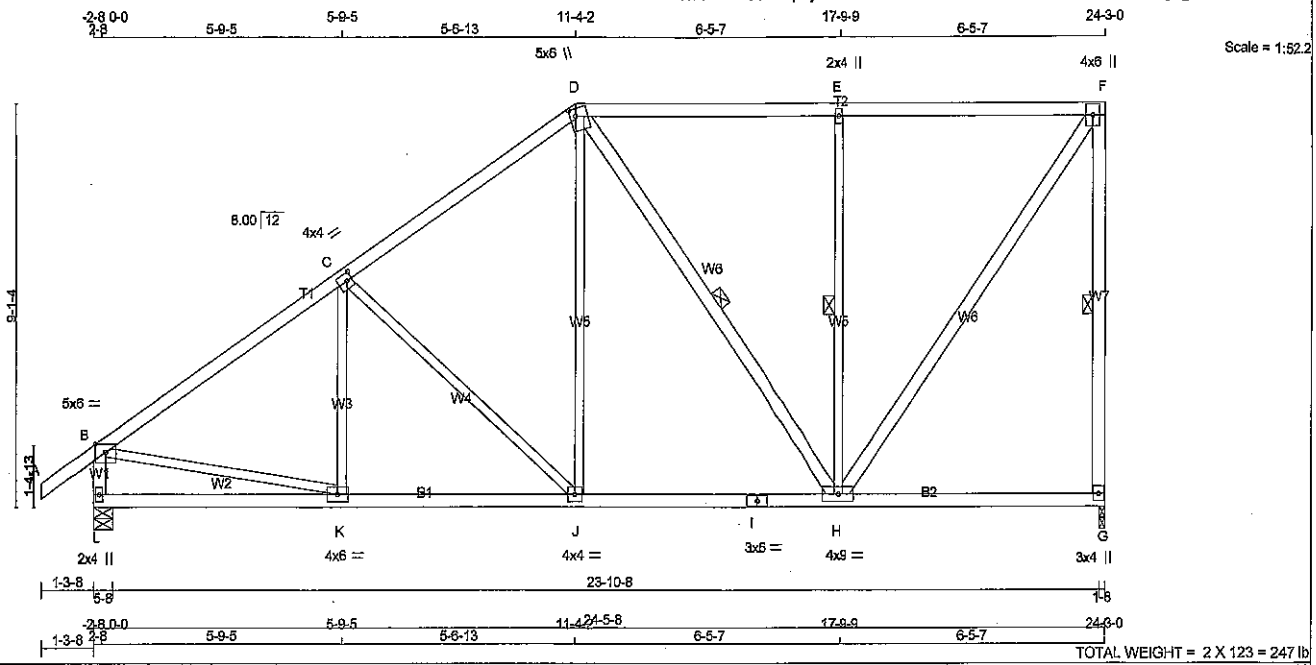

TOTAL WEIGHT = 2 X 123 = 247 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMW1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1365	0	1365	0	0	1-8	1-8
L	1493	0	1493	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1120	622 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
L	1207	700 / 0	255 / 0	0 / 0	0 / 0	252 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.67 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 35	-84.3	-84.3	0.14 (1)	10.00
B-C	-1502 / 0	-84.3	-84.3	0.57 (1)	4.67
C-D	-1187 / 0	-84.3	-84.3	0.53 (1)	5.18
D-E	-799 / 0	-84.3	-84.3	0.63 (1)	5.66
E-F	-799 / 0	-84.3	-84.3	0.63 (1)	5.66
G-F	-1292 / 0	0.0	0.0	0.50 (1)	5.68
L-K	0 / 0	-28.0	-28.0	0.22 (3)	10.00
K-J	0 / 1276	-28.0	-28.0	0.37 (2)	10.00
J-I	0 / 948	-28.0	-28.0	0.41 (2)	10.00
I-H	0 / 948	-28.0	-28.0	0.41 (2)	10.00
H-G	0 / 0	-28.0	-28.0	0.30 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K-C	-79 / 189	0.04 (3)
C-J	-447 / 0	0.52 (1)
J-D	0 / 480	0.11 (2)
D-H	-253 / 0	0.16 (1)
H-E	-575 / 0	0.36 (1)
H-F	0 / 1347	0.22 (1)
L-B	-1425 / 0	0.14 (1)
B-K	0 / 1302	0.29 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.63/1.00 (E-F:1) , BC=0.41/1.00 (H-J:2) , WB=0.52/1.00 (C-J:1) , SSI=0.26/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.32 (K) (INPUT = 1.00)

DWG NO. TAM3367-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285815	T46	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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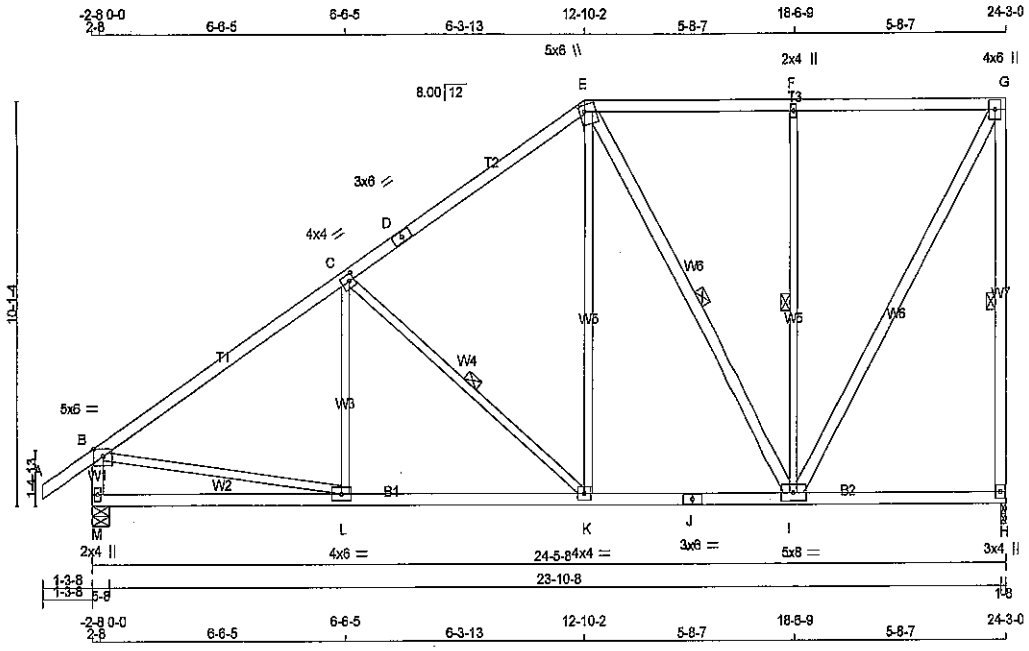

TOTAL WEIGHT = 3 X 129 = 388 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTWW+m	MT20	5.0	6.0	2.00	1.50
F	TMW+w	MT20	2.0	4.0		
G	TMVW+p	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWWW-t	MT20	5.0	8.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMW1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H	1365	0	1365	0	0	1-8	1-8
M	1493	0	1493	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1120	622 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
M	1207	700 / 0	255 / 0	0 / 0	0 / 0	252 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.74 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-H, C-K, E-I, F-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-84.3	-84.3	0.14 (1)	10.00
B- C	-1496 / 0	-84.3	-84.3	0.53 (1)	4.74
C- D	-1055 / 0	-84.3	-84.3	0.48 (1)	5.48
D- E	-1055 / 0	-84.3	-84.3	0.48 (1)	5.48
E- F	-853 / 0	-84.3	-84.3	0.35 (1)	6.25
F- G	-853 / 0	-84.3	-84.3	0.35 (1)	6.25
H- G	-1299 / 0	0.0	0.0	0.65 (1)	5.66
M- L	0 / 0	-28.0	-28.0	0.30 (3)	10.00
L- K	0 / 1275	-28.0	-28.0	0.45 (2)	10.00
K- J	0 / 851	-28.0	-28.0	0.32 (2)	10.00
J- I	0 / 851	-28.0	-28.0	0.32 (2)	10.00
I- H	0 / 0	-28.0	-28.0	0.22 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- C	-20 / 254	0.08 (3)
C- K	-568 / 0	0.28 (1)
K- E	0 / 535	0.12 (2)
E- I	-397 / 0	0.28 (1)
I- F	-595 / 0	0.42 (1)
I- G	0 / 1292	0.21 (1)
M- B	-1418 / 0	0.14 (1)
B- L	0 / 1295	0.29 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.65/1.00 (G-H:1) , BC=0.45/1.00 (K-L:2) , WB=0.42/1.00 (F-I:1) , SSI=0.23/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (B) (INPUT = 0.90)
JSI METAL= 0.32 (L) (INPUT = 1.00)

DWG NO. YAM 3368-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285814	T48A	6	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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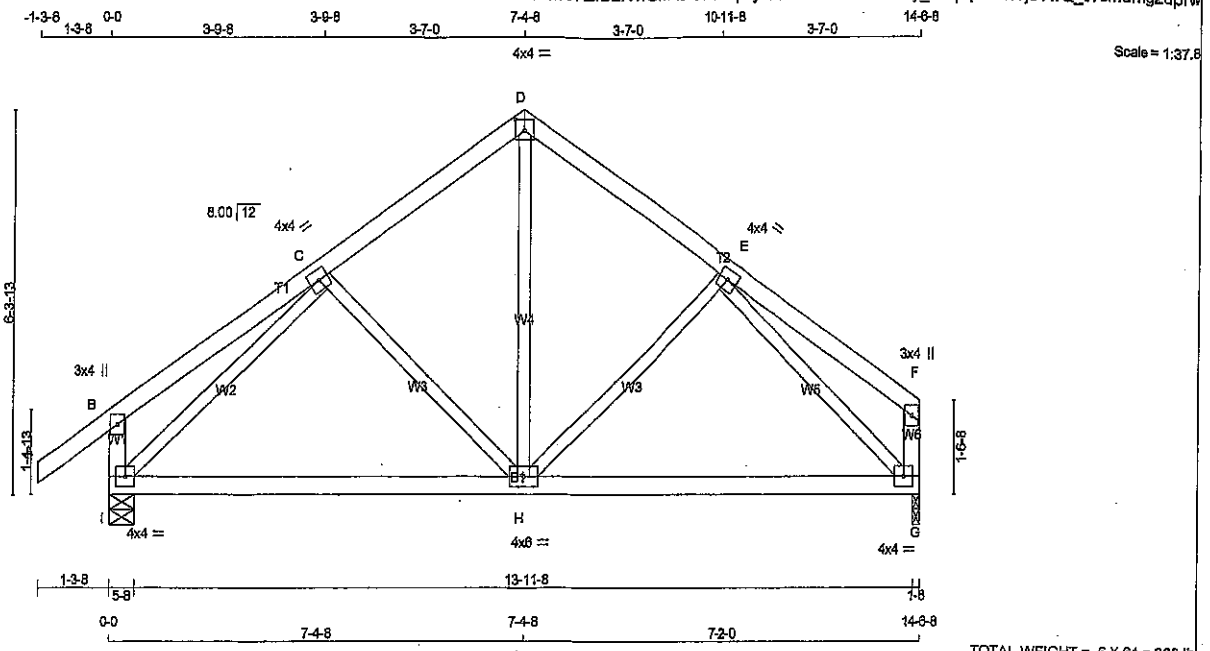


TOTAL WEIGHT = 6 X 61 = 366 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BMWWW-t	MT20	4.0	6.0		
I	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
I	932	0	932	0	0	5-8	5-8
G	616	0	616	0	0	1-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	749	442 / 0	153 / 0	0 / 0	0 / 0	154 / 0	0 / 0
G	670	372 / 0	153 / 0	0 / 0	0 / 0	145 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.3	-84.3	0.11 (1)		10.00
B- C	0 / 20	-84.3	-84.3	0.18 (1)		10.00
C- D	-649 / 0	-84.3	-84.3	0.14 (1)		6.25
D- E	-648 / 0	-84.3	-84.3	0.13 (1)		6.25
E- F	0 / 20	-84.3	-84.3	0.17 (1)		10.00
I- B	-235 / 0	0.0	0.0	0.02 (1)		7.81
G- F	-111 / 0	0.0	0.0	0.01 (1)		7.81
I- H	0 / 643	-28.0	-28.0	0.50 (2)		10.00
H- G	0 / 616	-28.0	-28.0	0.50 (2)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- D	0 / 455	0.10 (1)
H- E	-130 / 53	0.06 (1)
C- H	-164 / 42	0.07 (1)
I- C	-691 / 0	0.37 (1)
E- G	-662 / 0	0.34 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.46")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.46")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.18/1.00 (B-C:1) , BC=0.50/1.00 (H-I:2) , WB=0.37/1.00 (C-I:1) , SSI=0.17/1.00 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

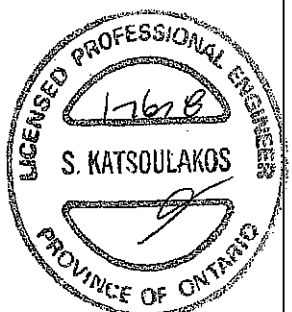
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (E) (INPUT = 0.90)
JSI METAL= 0.32 (C) (INPUT = 1.00)



DWG NO. TAM3983-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285812	T80	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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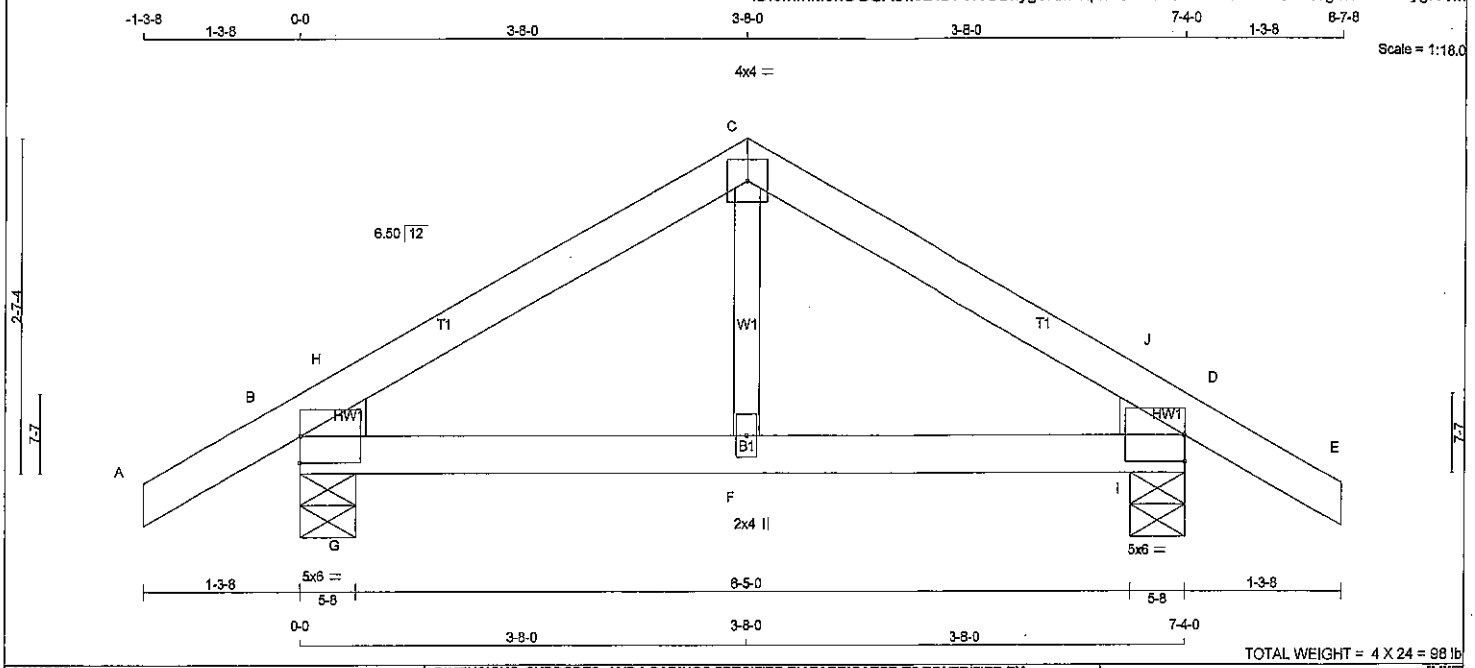

TOTAL WEIGHT = 4 X 24 = 98 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-I	MT20	5.0	6.0		
C	TTW-p	MT20	4.0	4.0		
D	TMBH1-I	MT20	5.0	6.0		
F	BMW+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG	HEEL
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	WEDGE
B	528	0	526	0	0	5-8	5-8	2x4 L
D	526	0	526	0	0	5-8	5-8	2x4 R

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	416	257 / 0	77 / 0	0 / 0	0 / 0	82 / 0	0 / 0
D	416	257 / 0	77 / 0	0 / 0	0 / 0	82 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
A- B	0 / 17	-84.3	-84.3	0.11 (1)	10.00	F- C	0 / 192	0.04 (2)	
B- H	-459 / 0	-84.3	-84.3	0.07 (1)	6.25	G- H	-55 / 132	0.00 (1)	
H- C	-410 / 0	-84.3	-84.3	0.11 (1)	6.25	I- J	-55 / 132	0.00 (1)	
C- J	-410 / 0	-84.3	-84.3	0.11 (1)	6.25				
J- D	-459 / 0	-84.3	-84.3	0.07 (1)	6.25				
D- E	0 / 17	-84.3	-84.3	0.11 (1)	10.00				
B- G	0 / 352	-28.0	-28.0	0.10 (1)	10.00				
G- F	0 / 352	-28.0	-28.0	0.13 (1)	10.00				
F- I	0 / 352	-28.0	-28.0	0.13 (1)	10.00				
I- D	0 / 352	-28.0	-28.0	0.10 (1)	10.00				

DWG NO. TAM 44980 17

STRUCTURAL

COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.8	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.24")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.24")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.11 (A-B:1) , BC=0.13 (F-G:1) , WB=0.04 (C-F:2) , SSI=0.11 (C-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (D) (INPUT = 0.90)
JSI METAL= 0.12 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285812	T80Z	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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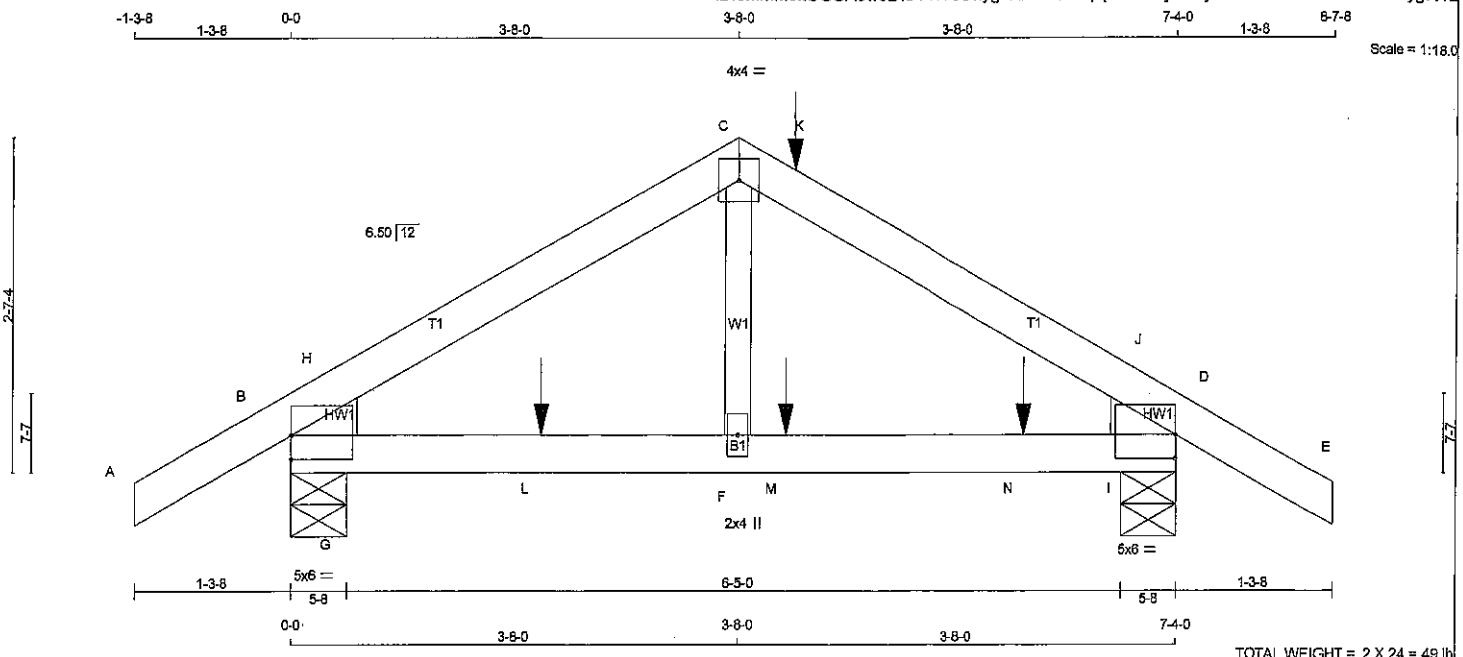

TOTAL WEIGHT = 2 X 24 = 49 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-I	MT20	5.0	6.0	2.25	
C	TTW-p	MT20	4.0	4.0		
D	TMBH1-I	MT20	5.0	6.0	2.25	
F	BMW+w	MT20	2.0	4.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 26.9 lbs FACTORED DOWN AT 4-0-12 ON TOP CHORD, AND 6.5 lbs FACTORED DOWN AT 2-0-12, AND 6.5 lbs FACTORED DOWN AT 4-0-12, AND 6.5 lbs FACTORED DOWN AT 6-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG	HEEL
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	WEDGE
B	543	0	543	0	0	5-8	5-8	2x4 L
D	547	0	547	0	0	5-8	5-8	2x4 R

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	430	265 / 0	81 / 0	0 / 0	0 / 0	85 / 0	0 / 0
D	434	266 / 0	82 / 0	0 / 0	0 / 0	86 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS							WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO							FR-TO		
A- B	0 / 17	-84.3	-84.3	0.12 (1)	10.00		F- C	0 / 203	0.05 (2)
B- H	-496 / 0	-84.3	-84.3	0.10 (1)	6.25		G- H	-43 / 141	0.00 (1)
H- C	-438 / 0	-84.3	-84.3	0.13 (1)	6.25		I- J	-50 / 142	0.00 (1)
C- K	-433 / 0	-84.3	-84.3	0.14 (1)	6.25				
K- J	-433 / 0	-84.3	-84.3	0.14 (1)	6.25				
J- D	-493 / 0	-84.3	-84.3	0.10 (1)	6.25				
D- E	0 / 17	-84.3	-84.3	0.12 (1)	10.00				
B- G	0 / 377	-28.0	-28.0	0.11 (1)	10.00				
G- L	0 / 377	-28.0	-28.0	0.15 (1)	10.00				
L- F	0 / 377	-28.0	-28.0	0.15 (1)	10.00				
F- M	0 / 377	-28.0	-28.0	0.15 (1)	10.00				
M- N	0 / 377	-28.0	-28.0	0.15 (1)	10.00				
N- I	0 / 377	-28.0	-28.0	0.15 (1)	10.00				
I- D	0 / 377	-28.0	-28.0	0.11 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
K	4-0-12	-27	-27	—	BACK	VERT	TOTAL
L	2-0-12	-4	-7	—	BACK	VERT	TOTAL
M	4-0-12	-4	-7	—	BACK	VERT	TOTAL
N	6-0-12	-4	-7	—	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.24")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.24")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.14 (C-J:1) , BC=0.15 (F-I:1) , WB=0.05 (C-F:2) , SSI=0.12 (C-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (D) (INPUT = 0.90)
JSI METAL= 0.12 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
9677
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 44981-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285813	T91	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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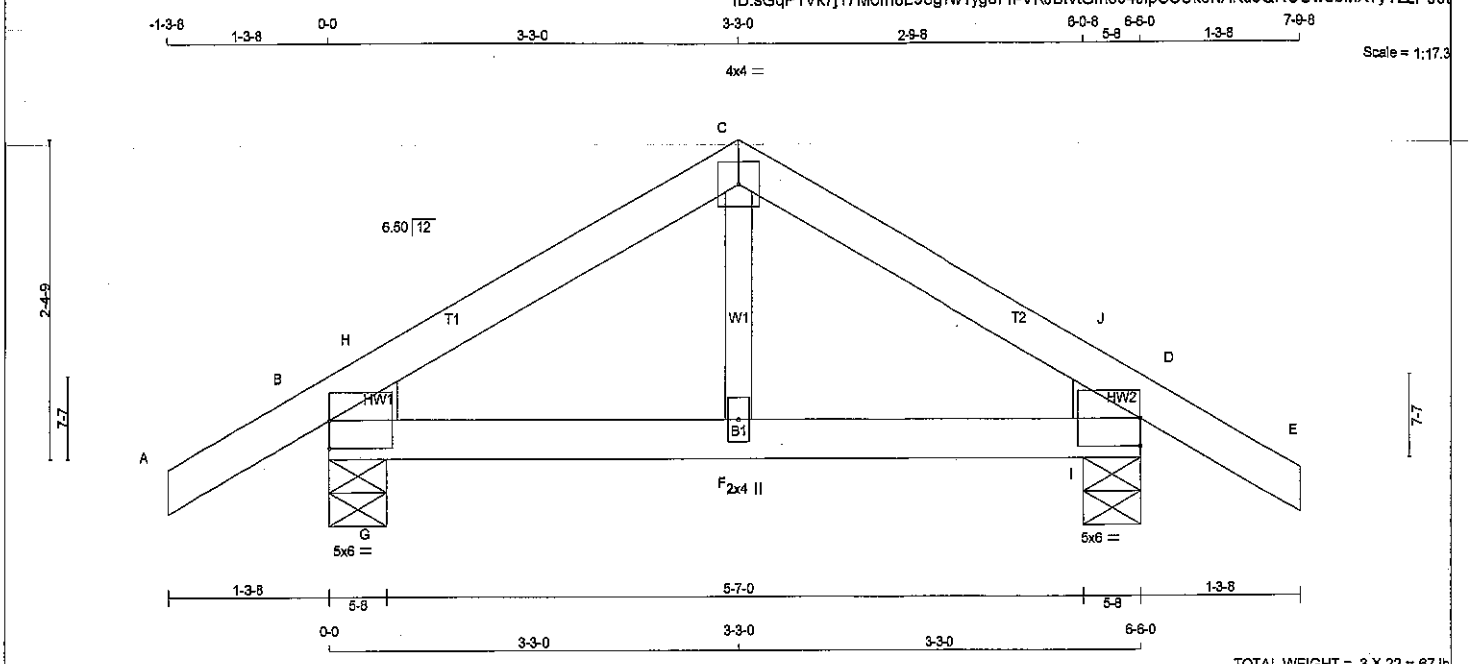

TOTAL WEIGHT = 3 X 22 = 67 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-I	MT20	5.0	6.0	2.50	
C	TTW-p	MT20	4.0	4.0		
D	TMBH1-I	MT20	5.0	6.0	2.50	
F	BMW+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
B	480	0	480	0	0	5-8	5-8	2x4 L
D	480	0	480	0	0	5-8	5-8	2x4 R

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	377	235 / 0	68 / 0	0 / 0	0 / 0	73 / 0	0 / 0
D	377	235 / 0	68 / 0	0 / 0	0 / 0	73 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 17	-84.3	-84.3	0.11 (1)	10.00	F- C	0 / 165	0.04 (2)
B- H	-411 / 0	-84.3	-84.3	0.05 (1)	6.25	G- H	-34 / 113	0.00 (1)
H- C	-355 / 0	-84.3	-84.3	0.08 (1)	6.25	I- J	-33 / 114	0.00 (1)
C- J	-355 / 0	-84.3	-84.3	0.08 (1)	6.25			
J- D	-412 / 0	-84.3	-84.3	0.05 (1)	6.25			
D- E	0 / 17	-84.3	-84.3	0.11 (1)	10.00			
B- G	0 / 305	-28.0	-28.0	0.08 (1)	10.00			
G- F	0 / 305	-28.0	-28.0	0.10 (1)	10.00			
F- I	0 / 305	-28.0	-28.0	0.10 (1)	10.00			
I- D	0 / 305	-28.0	-28.0	0.08 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.11/1.00 (A-B:1) , BC=0.10/1.00 (F-G:1) , WB=0.04/1.00 (C-F:2) , SSI=0.09/1.00 (C-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1887	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (D) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
41916
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 21232-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285813	G91	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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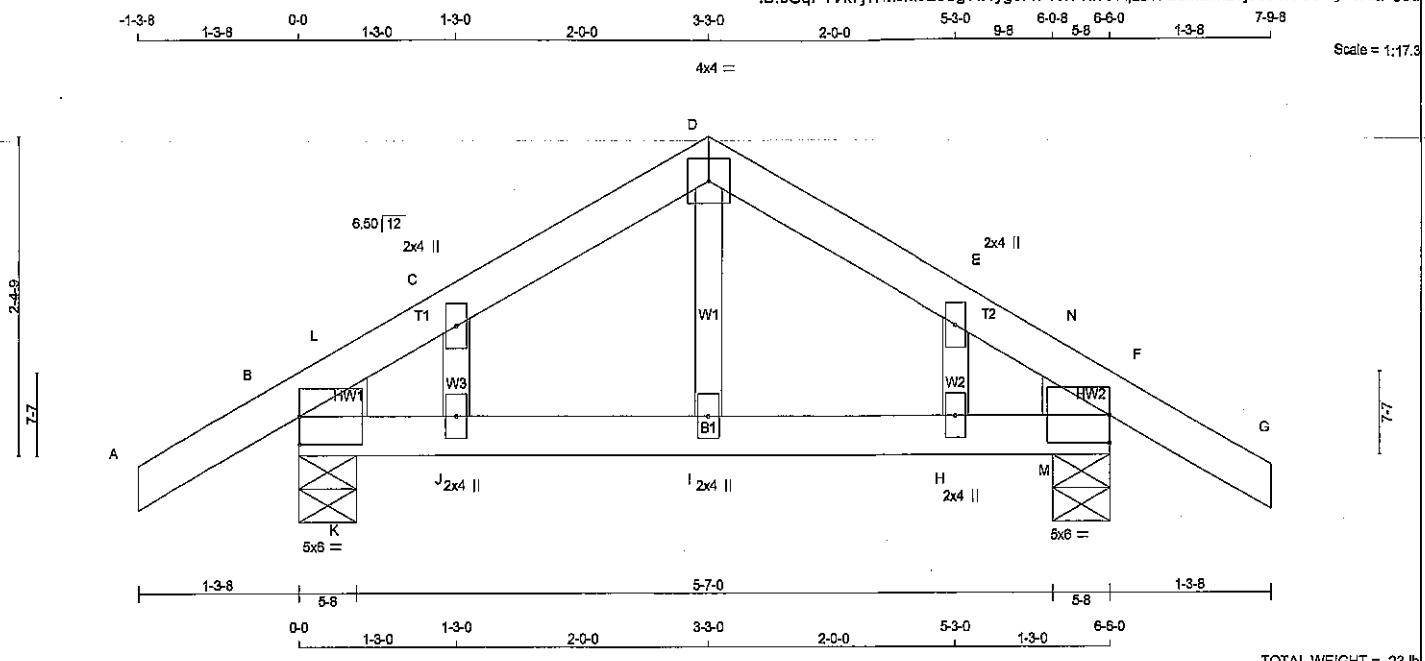

TOTAL WEIGHT = 23 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-I	MT20	5.0	6.0	2.50	
C	TMW+w	MT20	2.0	4.0		
D	TTW-p	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TMBH1-I	MT20	5.0	6.0	2.50	
H, I, J						
H	BMW+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
B	480	0	480	0	0	5-8	5-8	2x4 L
F	480	0	480	0	0	5-8	5-8	2x4 R

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	377	236 / 0	68 / 0	0 / 0	0 / 0	73 / 0	0 / 0
F	377	236 / 0	68 / 0	0 / 0	0 / 0	73 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 17	-84.3	-84.3	0.11 (1)	10.00
B- L	-525 / 0	-84.3	-84.3	0.11 (1)	6.25
L- C	-322 / 0	-84.3	-84.3	0.05 (1)	6.25
C- D	-298 / 0	-84.3	-84.3	0.06 (1)	6.25
D- E	-298 / 0	-84.3	-84.3	0.06 (1)	6.25
E- N	-322 / 0	-84.3	-84.3	0.05 (1)	6.25
N- F	-527 / 0	-84.3	-84.3	0.11 (1)	6.25
F- G	0 / 17	-84.3	-84.3	0.11 (1)	10.00
B- K	0 / 267	-28.0	-28.0	0.11 (1)	10.00
K- J	0 / 267	-28.0	-28.0	0.11 (1)	10.00
J- I	0 / 261	-28.0	-28.0	0.08 (1)	10.00
I- H	0 / 261	-28.0	-28.0	0.08 (1)	10.00
H- M	0 / 267	-28.0	-28.0	0.11 (1)	10.00
M- F	0 / 267	-28.0	-28.0	0.11 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- D	0 / 151	0.03 (2)
H- E	-81 / 26	0.01 (1)
J- C	-81 / 26	0.01 (1)
K- L	0 / 225	0.00 (1)
M- N	0 / 227	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.11/1.00 (F-N:1) , BC=0.11/1.00 (H-M:1) , WB=0.03/1.00 (D-I:2) , SSI=0.10/1.00 (F-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.66 (F) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)

DWG NO. TAM 21228-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285814	T152	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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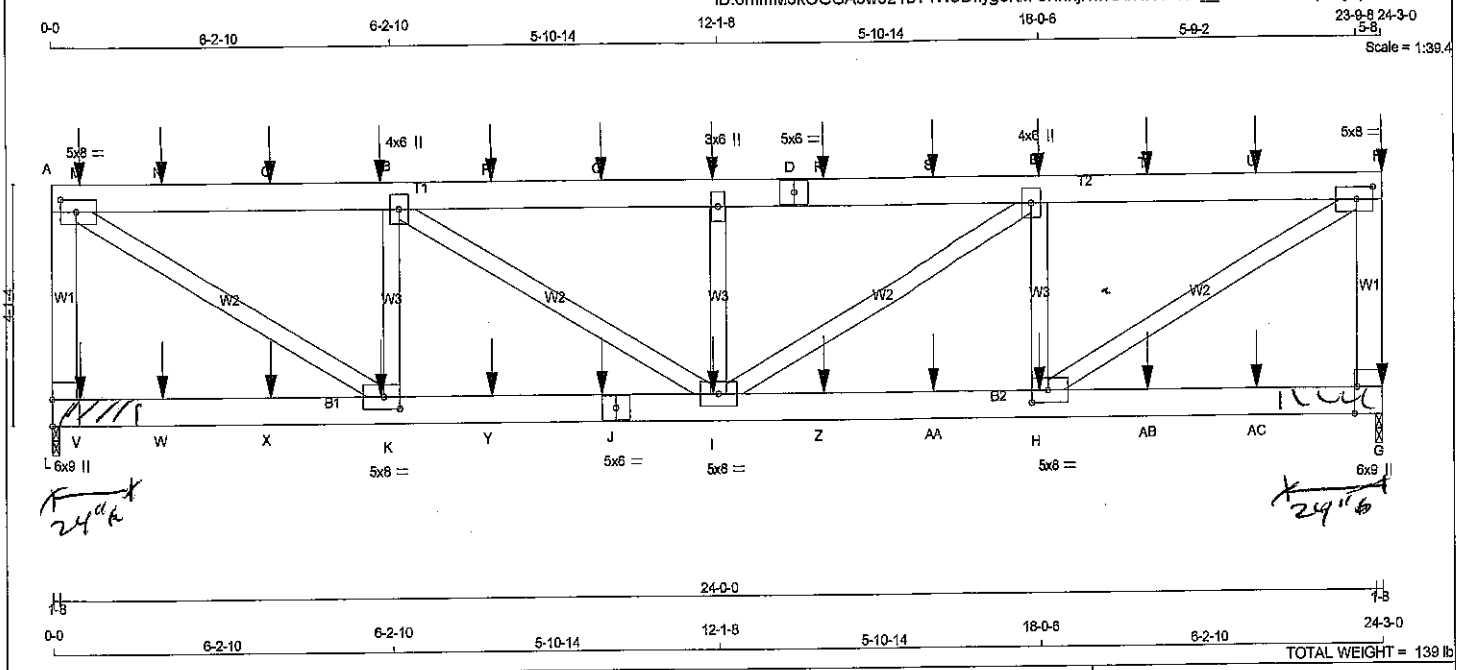


TOTAL WEIGHT = 139 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
L - A	2x6	DRY	No.2	SPF
A - D	2x6	DRY	2100F 1.8E	SPF
D - F	2x6	DRY	2100F 1.8E	SPF
G - F	2x6	DRY	No.2	SPF
L - J	2x6	DRY	2100F 1.8E	SPF
J - G	2x6	DRY	2100F 1.8E	SPF
ALL WEBS	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0	2.50	3.50
B	TMWW+t	MT20	4.0	6.0		
C	TMW+w	MT20	3.0	6.0		
D	TS-t	MT20	5.0	6.0		
E	TMWW+t	MT20	4.0	6.0		
F	TMVW-t	MT20	5.0	6.0	2.50	3.50
G	BMV1+p	MT20	6.0	9.0	Edge	0.50
H	BMWW-t	MT20	5.0	8.0	2.50	3.50
I	BMWWW-t	MT20	5.0	8.0		
J	BS-t	MT20	5.0	6.0		
K	BMWW-t	MT20	5.0	6.0	2.50	3.50
L	BMV1+p	MT20	6.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 121.6 lbs FACTORED DOWN AT 6-4, 101.4 lbs FACTOREO DOWN AT 2-0-4, 101.4 lbs FACTORED DOWN AT 4-0-4, 101.4 lbs FACTORED DOWN AT 6-0-4, 101.4 lbs FACTORED DOWN AT 8-0-4, 101.4 lbs FACTORED DOWN AT 10-0-4, 101.4 lbs FACTORED DOWN AT 12-0-4, 101.4 lbs FACTORED DOWN AT 14-0-4, 101.4 lbs FACTORED DOWN AT 16-0-4, 101.4 lbs FACTORED DOWN AT 18-0-4, 101.4 lbs FACTORED DOWN AT 20-0-4, AND 101.4 lbs FACTORED DOWN AT 22-0-4, AND 133.9 lbs FACTORED DOWN AT 24-3-0 ON TOP CHORD, AND 81.7 lbs FACTORED DOWN AT 6-4, 69.9 lbs FACTORED DOWN AT 2-0-4, 69.9 lbs FACTORED DOWN AT 4-0-4, 69.9 lbs FACTORED DOWN AT 6-0-4, 69.9 lbs FACTORED DOWN AT 8-0-4, 69.9 lbs FACTORED DOWN AT 10-0-4, 69.9 lbs FACTORED DOWN AT 12-0-4, 69.9 lbs FACTORED DOWN AT 14-0-4, 69.9 lbs FACTORED DOWN AT 16-0-4, 69.9 lbs FACTORED DOWN AT 18-0-4, 89.9 lbs FACTORED DOWN AT 20-0-4, AND 69.9 lbs FACTORED DOWN AT 22-0-4, AND 88.8 lbs FACTORED DOWN AT 24-3-0 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	2308	0	2308	0	0	1-8	1-8
G	2322	0	2322	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1912	1034 / 0	454 / 0	0 / 0	0 / 0	424 / 0	0 / 0
G	1924	1040 / 0	456 / 0	0 / 0	0 / 0	427 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.10 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
L- A	-2155 / 0	0.0	0.0	0.31 (1)	6.90
A- M	-2985 / 0	-84.3	-84.3	0.21 (1)	5.58
M- N	-2985 / 0	-84.3	-84.3	0.21 (1)	5.58
N- O	-2985 / 0	-84.3	-84.3	0.21 (1)	5.58
O- B	-2985 / 0	-84.3	-84.3	0.21 (1)	5.58
B- P	-3779 / 0	-84.3	-84.3	0.22 (1)	5.10
P- Q	-3779 / 0	-84.3	-84.3	0.22 (1)	5.10
Q- C	-3779 / 0	-84.3	-84.3	0.22 (1)	5.10
C- D	-3779 / 0	-84.3	-84.3	0.22 (1)	5.10
D- R	-3779 / 0	-84.3	-84.3	0.22 (1)	5.10
R- S	-3779 / 0	-84.3	-84.3	0.22 (1)	5.10
S- E	-3779 / 0	-84.3	-84.3	0.22 (1)	5.10
E- T	-2969 / 0	-84.3	-84.3	0.20 (1)	5.61
T- U	-2969 / 0	-84.3	-84.3	0.20 (1)	5.61
U- F	-2969 / 0	-84.3	-84.3	0.20 (1)	5.61
G- F	-2164 / 0	0.0	0.0	0.32 (1)	6.89
L- V	0 / 0	-28.0	-28.0	0.12 (2)	10.00
V- W	0 / 0	-28.0	-28.0	0.12 (2)	10.00
W- X	0 / 0	-28.0	-28.0	0.12 (2)	10.00
X- K	0 / 0	-28.0	-28.0	0.12 (2)	10.00
K- Y	0 / 2985	-28.0	-28.0	0.23 (2)	10.00
Y- J	0 / 2985	-28.0	-28.0	0.23 (2)	10.00
J- I	0 / 2985	-28.0	-28.0	0.23 (2)	10.00
I- Z	0 / 2969	-28.0	-28.0	0.22 (2)	10.00
Z-AA	0 / 2969	-28.0	-28.0	0.22 (2)	10.00
AA- H	0 / 2969	-28.0	-28.0	0.22 (2)	10.00
H-AB	0 / 0	-28.0	-28.0	0.11 (3)	10.00
AB-AC	0 / 0	-28.0	-28.0	0.11 (3)	10.00
AC- G	0 / 0	-28.0	-28.0	0.11 (3)	10.00

WEBS FR-TO	MAX. FACTORED MEMB. FORCE (LBS)	MAX CSI (LC)
H- F	0 / 3441	0.61 (1)
A- K	0 / 3460	0.61 (1)
H- E	-1420 / 0	0.24 (1)
K- B	-1426 / 0	0.24 (1)
I- E	0 / 952	0.17 (1)
B- I	0 / 933	0.16 (1)
I- C	-730 / 0	0.12 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
B	6-0-4	-101	-101	---	FRONT	VERT	TOTAL
C	12-0-4	-101	-101	---	FRONT	VERT	TOTAL
E	18-0-4	-101	-101	---	FRONT	VERT	TOTAL
F	24-3-0	-134	-134	---	FRONT	VERT	TOTAL
G	24-3-0	-51	-89	---	FRONT	VERT	TOTAL
H	18-0-4	-40	-70	---	FRONT	VERT	TOTAL

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.32/1.00 (F-G:1) , BC=0.23/1.00 (I-K:2) , WB=0.61/1.00 (A-K:1) , SSI=0.29/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

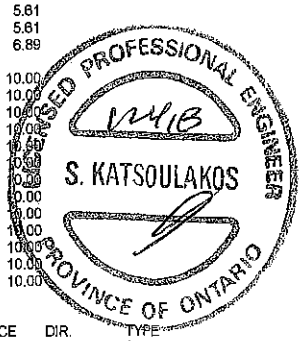
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (F) (INPUT = 0.90)
JSI METAL= 0.56 (F) (INPUT = 1.00).



DWG NO. TAM5077-18
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285814	T152	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
I	12-0-4	-40	-70	--	FRONT	VERT	TOTAL
J	10-0-4	-40	-70	--	FRONT	VERT	TOTAL
K	8-0-4	-40	-70	--	FRONT	VERT	TOTAL
M	6-4	-122	-122	--	FRONT	VERT	TOTAL
N	2-0-4	-101	-101	--	FRONT	VERT	TOTAL
O	4-0-4	-101	-101	--	FRONT	VERT	TOTAL
P	8-0-4	-101	-101	--	FRONT	VERT	TOTAL
Q	10-0-4	-101	-101	--	FRONT	VERT	TOTAL
R	14-0-4	-101	-101	--	FRONT	VERT	TOTAL
S	16-0-4	-101	-101	--	FRONT	VERT	TOTAL
T	20-0-4	-101	-101	--	FRONT	VERT	TOTAL
U	22-0-4	-101	-101	--	FRONT	VERT	TOTAL
V	6-4	-47	-62	--	FRONT	VERT	TOTAL
W	2-0-4	-40	-70	--	FRONT	VERT	TOTAL
X	4-0-4	-40	-70	--	FRONT	VERT	TOTAL
Y	8-0-4	-40	-70	--	FRONT	VERT	TOTAL
Z	14-0-4	-40	-70	--	FRONT	VERT	TOTAL
AA	18-0-4	-40	-70	--	FRONT	VERT	TOTAL
AB	20-0-4	-40	-70	--	FRONT	VERT	TOTAL
AC	22-0-4	-40	-70	--	FRONT	VERT	TOTAL

SCAB JOINT # L & # G WITH 2 X 6 #2 SPF 24' LONG 1 SIDE(S) USING 2 ROW(S) 3½" ARDOX SPIRAL/~~COMMON WIRE~~ NAILS AT 2" O/C STAGGERED (18 NAILS/SCAB). REQUIRED FOR 1½" BRG PROVISION.

LICENSED PROFESSIONAL ENGINEER
1418
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM5078-B
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285814	T152Z	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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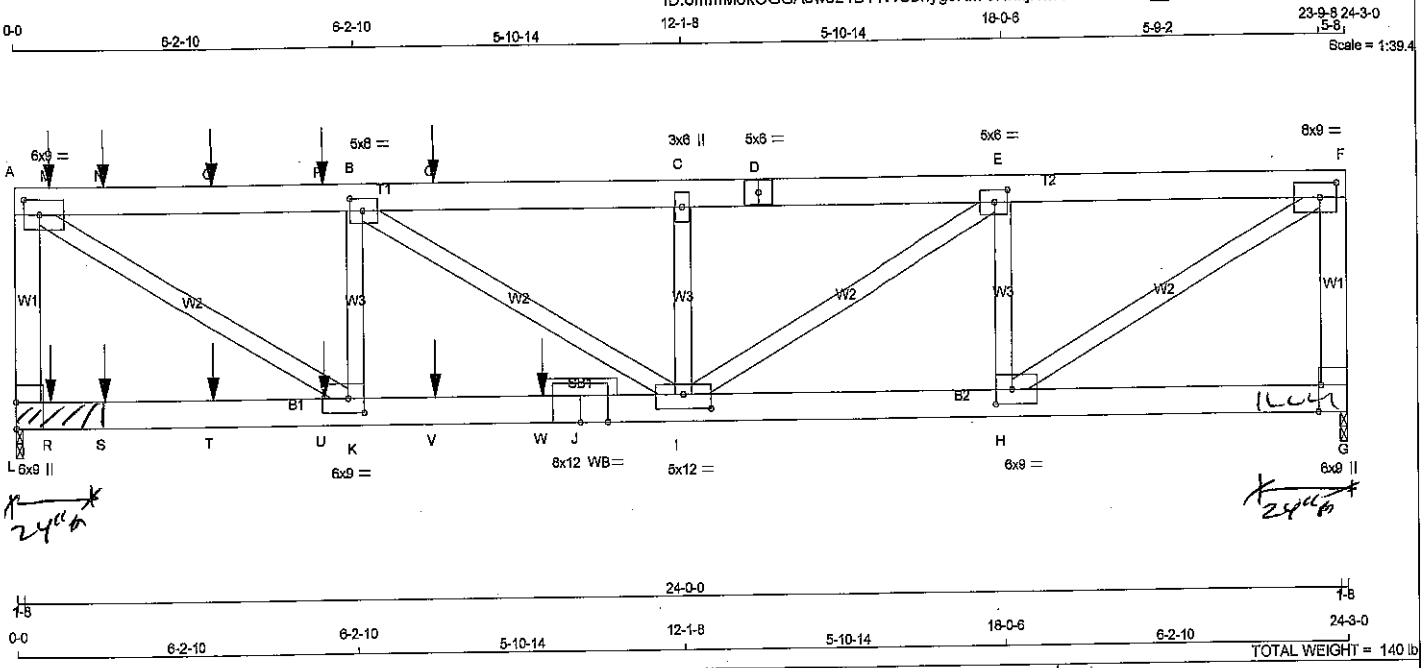

TOTAL WEIGHT = 140 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
L - A	2x6	DRY	No.2	SPF
A - D	2x6	DRY	2100F 1.8E	SPF
D - F	2x6	DRY	2100F 1.8E	SPF
G - F	2x6	DRY	No.2	SPF
L - J	2x6	DRY	2100F 1.8E	SPF
J - G	2x6	DRY	2100F 1.8E	SPF
ALL WEBS	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-t	MT20	6.0	9.0	3.00	3.50
B	TMWW-t	MT20	5.0	6.0	2.50	2.75
C	TMW+w	MT20	3.0	6.0		
D	TS-t	MT20	5.0	6.0		
E	TMWW-t	MT20	5.0	8.0	2.50	2.75
F	TMWW-t	MT20	6.0	9.0	3.00	3.50
G	BMV1+p	MT20	6.0	9.0	Edge	0.50
H	BMWW-t	MT20	6.0	9.0	3.00	3.50
I	BMWWW-t	MT20	5.0	12.0	3.00	6.00
J	BS-t	MT20	8.0	12.0		
K	BMWW-t	MT20	6.0	9.0	3.00	3.50
L	BMV1+p	MT20	6.0	9.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

WB - INDICATES BLOCKING REQUIRED

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 116.3 lbs FACTORED DOWN AT 7-12, 101.4 lbs FACTORED DOWN AT 1-7-12, 101.4 lbs FACTORED DOWN AT 3-7-12, AND 101.4 lbs FACTORED DOWN AT 5-7-12, AND 101.4 lbs FACTORED DOWN AT 7-7-12 ON TOP CHORD, AND 78.6 lbs FACTORED DOWN AT 7-12, 69.9 lbs FACTORED DOWN AT 1-7-12, 69.9 lbs FACTORED DOWN AT 3-7-12, 69.9 lbs FACTORED DOWN AT 5-7-12, AND 69.9 lbs FACTORED DOWN AT 7-7-12, AND 1637.7 lbs FACTORED DOWN AT 9-7-0 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

SCAB JOINT # L & # G WITH 2 X 6 #2 SPF 24" LONG 1 SIDE(S) USING 2 ROW(S) 3¼" ARDOX SPIRAL/~~COMMON WIRE~~ NAILS AT 2" O/C STAGGERED (16 NAILS/SCAB). REQUIRED FOR 1½" BRG PROVISION.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	2966	0	2966	0	0	1-8	1.5
G	2121	0	2121	0	0	1-8	1.5

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	2415	1373 / 0	526 / 0	0 / 0	0 / 0	513 / 0	0 / 0
G	1723	986 / 0	372 / 0	0 / 0	0 / 0	364 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.55 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
L-A	-2901 / 0	0.0	0.0	0.42 (1)	6.12
A-M	-4224 / 0	-84.3	-84.3	0.25 (1)	4.85
M-N	-4224 / 0	-84.3	-84.3	0.25 (1)	4.85
N-O	-4224 / 0	-84.3	-84.3	0.25 (1)	4.85
O-P	-4224 / 0	-84.3	-84.3	0.25 (1)	4.85
P-B	-4224 / 0	-84.3	-84.3	0.25 (1)	4.85
B-Q	-5023 / 0	-84.3	-84.3	0.24 (1)	4.55
Q-C	-5023 / 0	-84.3	-84.3	0.24 (1)	4.55
C-D	-5023 / 0	-84.3	-84.3	0.18 (1)	4.61
D-E	-5023 / 0	-84.3	-84.3	0.18 (1)	4.61
E-F	-3067 / 0	-84.3	-84.3	0.14 (1)	5.62
G-F	-2014 / 0	0.0	0.0	0.29 (1)	7.06
L-R	0 / 0	-28.0	-28.0	0.28 (1)	10.00
R-S	0 / 0	-28.0	-28.0	0.28 (1)	10.00
S-T	0 / 0	-28.0	-28.0	0.28 (1)	10.00
T-U	0 / 0	-28.0	-28.0	0.28 (1)	10.00
U-K	0 / 0	-28.0	-28.0	0.28 (1)	10.00
K-V	0 / 4224	-28.0	-28.0	0.93 (1)	10.00
V-W	0 / 4224	-28.0	-28.0	0.93 (1)	10.00
W-J	0 / 4224	-28.0	-28.0	0.93 (1)	10.00
J-I	0 / 4224	-28.0	-28.0	0.93 (1)	10.00
I-H	0 / 3087	-28.0	-28.0	0.47 (1)	10.00
H-G	0 / 0	-28.0	-28.0	0.09 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H-F	0 / 3556	0.63 (1)
A-K	0 / 4898	0.87 (1)
H-E	-1800 / 0	0.31 (1)
K-B	-1395 / 0	0.24 (1)
I-E	0 / 2298	0.41 (1)
B-I	0 / 939	0.17 (1)
I-C	-431 / 0	0.07 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
M	7-12	-116	-116	---	BACK	VERT	TOTAL
N	1-7-12	-101	-101	---	BACK	VERT	TOTAL
O	3-7-12	-101	-101	---	BACK	VERT	TOTAL
P	5-7-12	-101	-101	---	BACK	VERT	TOTAL
Q	7-7-12	-101	-101	---	BACK	VERT	TOTAL
R	7-12	-45	-79	---	BACK	VERT	TOTAL
S	1-7-12	-40	-70	---	BACK	VERT	TOTAL
T	3-7-12	-40	-70	---	BACK	VERT	TOTAL
U	5-7-12	-40	-70	---	BACK	VERT	TOTAL
V	7-7-12	-40	-70	---	BACK	VERT	TOTAL
W	9-7-0	-1638	-1638	---	BACK	VERT	TOTAL

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.28")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 716 (0.41")

CSI: TC=0.42/1.00 (A-L:1) , BC=0.93/1.00 (I-K:1) , WB=0.87/1.00 (A-K:1) , SSI=0.69/1.00 (I-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (F) (INPUT = 0.90)
JSI METAL= 0.94 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
PROVINCE OF ONTARIO
12/18
S. KATSOULAKOS

DWG NO. TAM 50798
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285814	T153	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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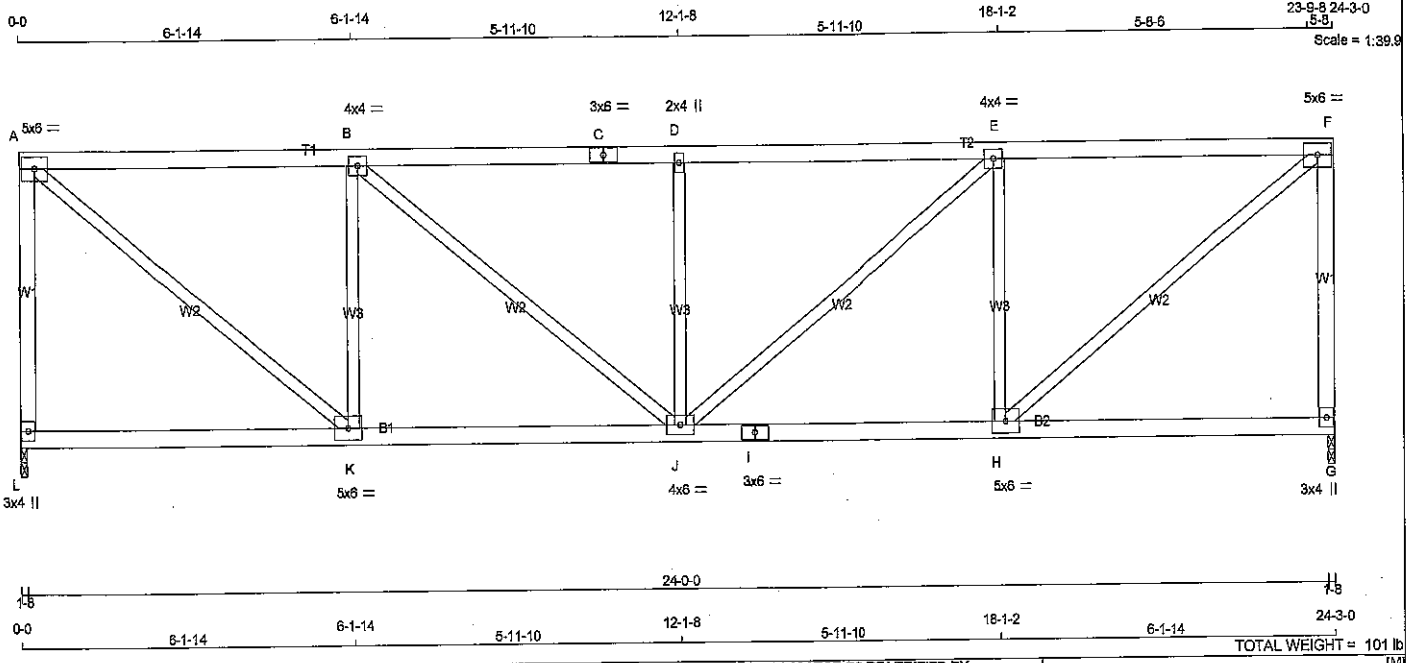

TOTAL WEIGHT = 101 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
L - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0		
B	TMWW-t	MT20	4.0	4.0		
C	TS-t	MT20	3.0	6.0		
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMVW-t	MT20	5.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	5.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
L	1361	0	1361	0	0	1-8	1-8
G	1361	0	1361	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1118	620 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
G	1118	620 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.36 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L-A	-1291 / 0	0.0	0.0	0.57 (1)	7.10	H-F	0 / 1762	0.40 (1)
A-B	-1388 / 0	-84.3	-84.3	0.53 (1)	4.84	A-K	0 / 1762	0.40 (1)
B-C	-1768 / 0	-84.3	-84.3	0.58 (1)	4.36	H-E	-891 / 0	0.34 (1)
C-D	-1768 / 0	-84.3	-84.3	0.58 (1)	4.36	K-B	-891 / 0	0.34 (1)
D-E	-1768 / 0	-84.3	-84.3	0.56 (1)	4.36	J-E	0 / 488	0.11 (1)
E-F	-1366 / 0	-84.3	-84.3	0.53 (1)	4.84	B-J	0 / 488	0.11 (1)
G-F	-1291 / 0	0.0	0.0	0.57 (1)	7.10	J-D	-460 / 0	0.18 (1)
L-K	0 / 0	-28.0	-28.0	0.26 (3)	10.00			
K-J	0 / 1388	-28.0	-28.0	0.42 (2)	10.00			
J-I	0 / 1388	-28.0	-28.0	0.42 (2)	10.00			
I-H	0 / 1388	-28.0	-28.0	0.42 (2)	10.00			
H-G	0 / 0	-28.0	-28.0	0.26 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.57/1.00 (A-L:1) , BC=0.42/1.00 (J-K:2) , WB=0.40/1.00 (F-H:1) , SSI=0.24/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (K) (INPUT = 0.90)
JSI METAL= 0.39 (I) (INPUT = 1.00)

DWG NO. TAM 5079 -18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285814	T154	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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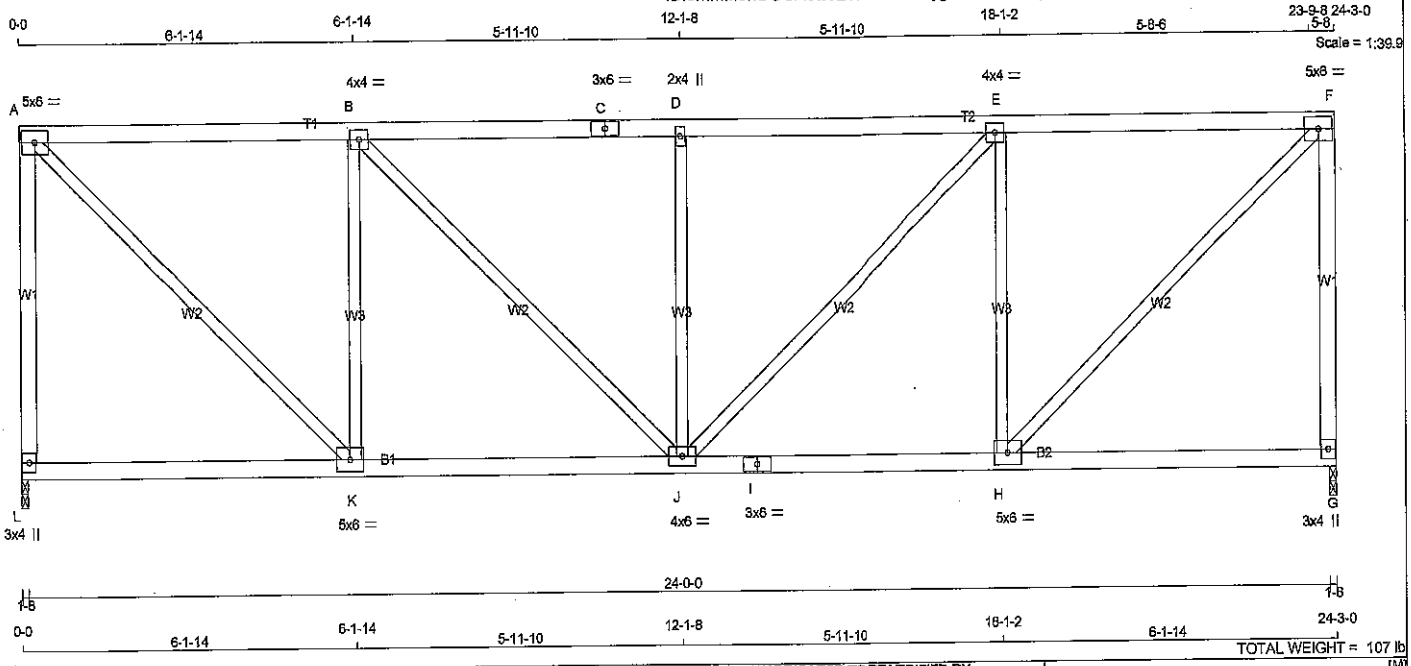

TOTAL WEIGHT = 107 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
L - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0		
B	TMWW-t	MT20	4.0	4.0		
C	TS-t	MT20	3.0	6.0		
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMVW-t	MT20	5.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	5.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1361	0	1361	0	0	1-8	1-8
G	1361	0	1361	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1118	620 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
G	1118	620 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.72 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L-A	-1292 / 0	0.0	0.0	0.93 (1)	7.10	H-F	0 / 1581	0.36 (1)
A-B	-1150 / 0	-84.3	-84.3	0.51 (1)	5.22	A-K	0 / 1581	0.36 (1)
B-C	-1464 / 0	-84.3	-84.3	0.53 (1)	4.72	H-E	-891 / 0	0.52 (1)
C-D	-1464 / 0	-84.3	-84.3	0.53 (1)	4.72	K-B	-891 / 0	0.52 (1)
D-E	-1464 / 0	-84.3	-84.3	0.53 (1)	4.72	J-E	0 / 438	0.10 (1)
E-F	-1150 / 0	-84.3	-84.3	0.51 (1)	5.22	B-J	0 / 438	0.10 (1)
G-F	-1292 / 0	0.0	0.0	0.93 (1)	7.10	J-D	-460 / 0	0.27 (1)
L-K	0 / 0	-28.0	-28.0	0.26 (3)	10.00			
K-J	0 / 1150	-28.0	-28.0	0.39 (2)	10.00			
J-I	0 / 1150	-28.0	-28.0	0.39 (2)	10.00			
I-H	0 / 1150	-28.0	-28.0	0.39 (2)	10.00			
H-G	0 / 0	-28.0	-28.0	0.26 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.93/1.00 (A-L:1) , BC=0.39/1.00 (J-K:2) , WB=0.52/1.00 (E-H:1) , SSI=0.24/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (F) (INPUT = 0.90)
JSI METAL= 0.33 (I) (INPUT = 1.00)

DWG NO. TAM 5080 -18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285814	T155	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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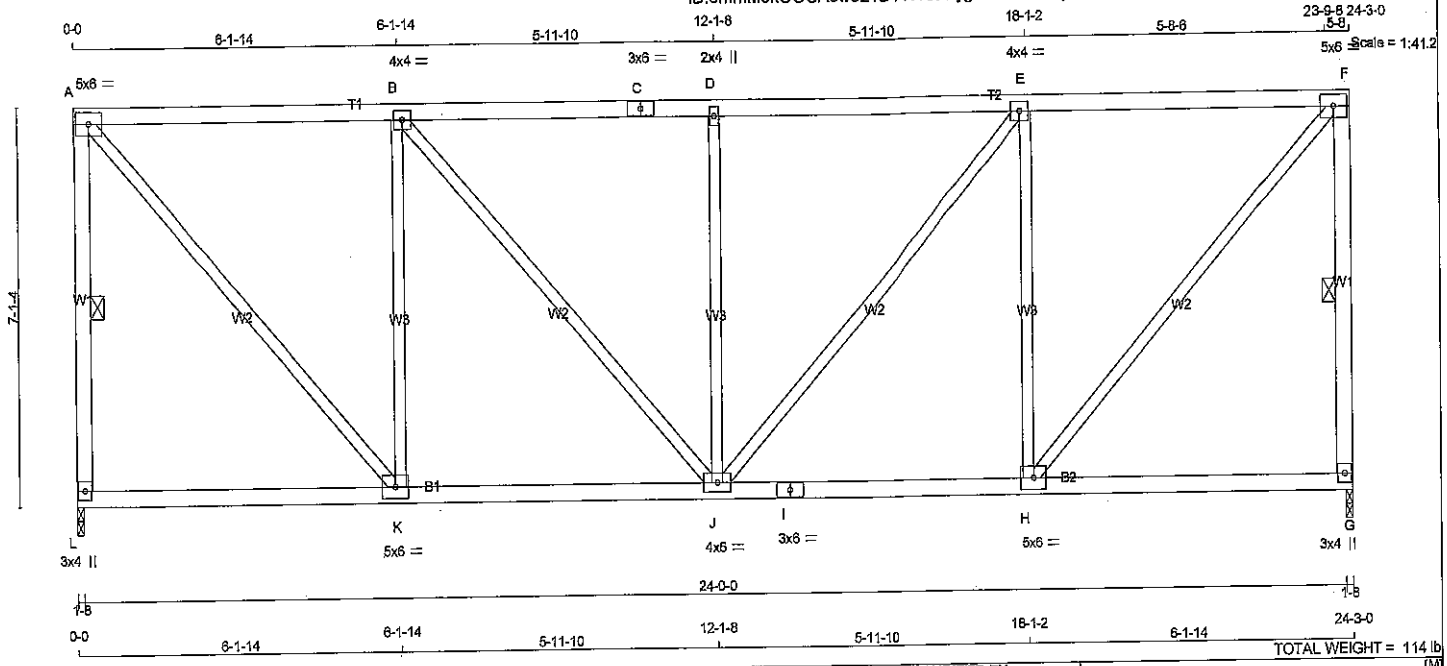

TOTAL WEIGHT = 114 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
L - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0		
B	TMWW-t	MT20	4.0	4.0		
C	TS-t	MT20	3.0	6.0		
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMVW-t	MT20	5.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	5.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1361	0	1361	0	0	1-8	1-8
G	1361	0	1361	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1118	620 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
G	1118	620 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.04 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF A-L, F-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- A	-1292 / 0	0.0	0.0	0.29 (1)	5.68	H- F	0 / 1464	0.33 (1)
A- B	-982 / 0	-84.3	-84.3	0.50 (1)	5.55	A- K	0 / 1464	0.33 (1)
B- C	-1249 / 0	-84.3	-84.3	0.52 (1)	5.04	H- E	-891 / 0	0.78 (1)
C- D	-1249 / 0	-84.3	-84.3	0.52 (1)	5.04	K- B	-891 / 0	0.78 (1)
D- E	-1249 / 0	-84.3	-84.3	0.52 (1)	5.04	J- E	0 / 406	0.09 (1)
E- F	-982 / 0	-84.3	-84.3	0.50 (1)	5.55	B- J	0 / 406	0.09 (1)
G- F	-1292 / 0	0.0	0.0	0.29 (1)	5.68	J- D	-460 / 0	0.40 (1)
L- K	0 / 0	-28.0	-28.0	0.26 (3)	10.00			
K- J	0 / 982	-28.0	-28.0	0.37 (2)	10.00			
J- I	0 / 982	-28.0	-28.0	0.37 (2)	10.00			
I- H	0 / 982	-28.0	-28.0	0.37 (2)	10.00			
H- G	0 / 0	-28.0	-28.0	0.26 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.52/1.00 (B-D:1) , BC=0.37/1.00 (H-J:2) , WB=0.78/1.00 (E-H:1) , SSI=0.24/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (F) (INPUT = 0.90)
JSI METAL= 0.29 (I) (INPUT = 1.00)

DWG NO. TAM5001 -18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285814	T156	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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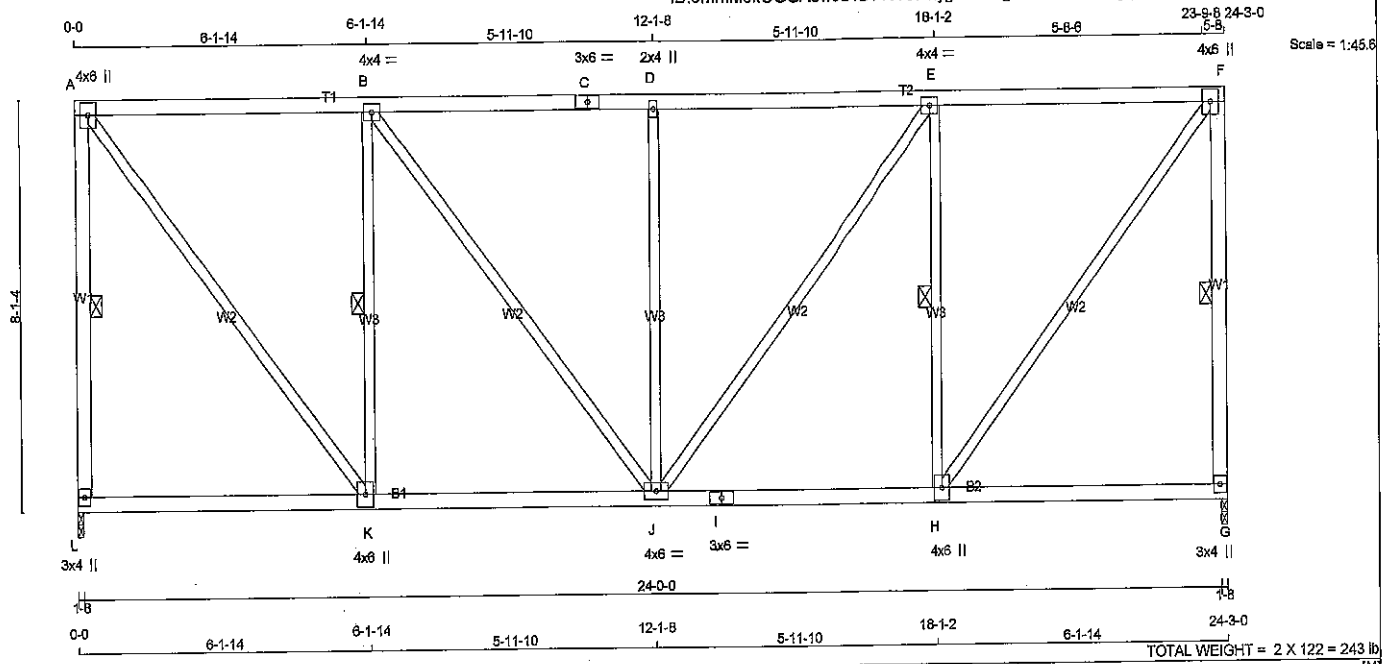

TOTAL WEIGHT = 2 X 122 = 243 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
L - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0		
B	TMWW-t	MT20	4.0	4.0		
C	TS-t	MT20	3.0	6.0		
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWW+t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BMWW+t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1361	0	1361	0	0	1-8	1-8
G	1361	0	1361	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1118	620 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
G	1118	620 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.33 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF A-L, F-G, E-H, B-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- A	-1292 / 0	0.0	0.0	0.38 (1)	5.68	H- F	0 / 1383	0.31 (1)
A- B	-856 / 0	-84.3	-84.3	0.49 (1)	5.85	A- K	0 / 1383	0.31 (1)
B- C	-1089 / 0	-84.3	-84.3	0.51 (1)	5.33	H- E	-891 / 0	0.37 (1)
C- D	-1089 / 0	-84.3	-84.3	0.51 (1)	5.33	K- B	-891 / 0	0.37 (1)
D- E	-1089 / 0	-84.3	-84.3	0.51 (1)	5.33	J- E	0 / 384	0.09 (1)
E- F	-856 / 0	-84.3	-84.3	0.49 (1)	5.85	B- J	0 / 384	0.09 (1)
G- F	-1292 / 0	0.0	0.0	0.38 (1)	5.68	J- D	-460 / 0	0.58 (1)
L- K	0 / 0	-28.0	-28.0	0.26 (3)	10.00			
K- J	0 / 856	-28.0	-28.0	0.36 (2)	10.00			
J- I	0 / 858	-28.0	-28.0	0.36 (2)	10.00			
I- H	0 / 856	-28.0	-28.0	0.36 (2)	10.00			
H- G	0 / 0	-28.0	-28.0	0.26 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.51/1.00 (D-E:1) , BC=0.36/1.00 (H-J:2) , WB=0.58/1.00 (D-J:1) , SSI=0.24/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (K) (INPUT = 0.90)
JSI METAL= 0.28 (A) (INPUT = 1.00)

DWG NO. TAM5082-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43854	DRWG NO.
285814	T157	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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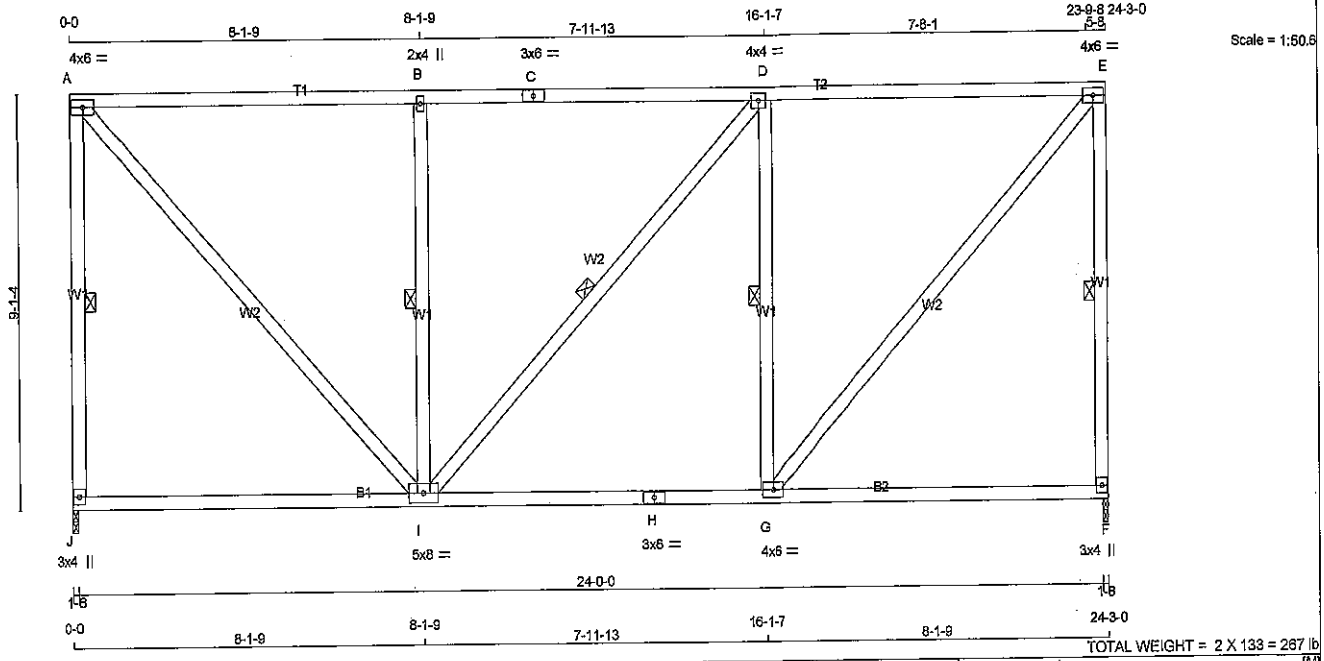


TOTAL WEIGHT = 2 X 133 = 267 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
J - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0		
B	TMW+w	MT20	2.0	4.0		
C	TS-t	MT20	3.0	6.0		
D	TMWW-t	MT20	4.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWW-t	MT20	4.0	6.0		
H	BS-t	MT20	3.0	8.0		
I	BMWWW-t	MT20	5.0	8.0		
J	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	1361	0	1361	0	0	1-8	1-8
F	1361	0	1361	0	0	1-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	1118	620 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
F	1118	620 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.61 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

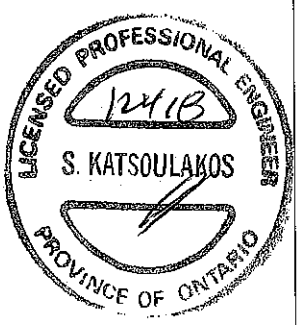
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF A-J, E-F, B-I, D-I, D-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- A	-1269 / 0	0.0	0.0	0.49 (1)	5.72	A- I	0 / 1351	0.22 (1)	
A- B	-916 / 0	-84.3	-84.3	0.86 (1)	4.62	I- B	-745 / 0	0.29 (1)	
B- C	-916 / 0	-84.3	-84.3	0.86 (1)	4.81	I- D	0 / 0	0.00 (1)	
C- D	-916 / 0	-84.3	-84.3	0.86 (1)	4.81	G- D	-746 / 0	0.29 (1)	
D- E	-917 / 0	-84.3	-84.3	0.86 (1)	4.81	G- E	0 / 1352	0.22 (1)	
F- E	-1269 / 0	0.0	0.0	0.49 (1)	5.72				
J- I	0 / 0	-28.0	-28.0	0.43 (3)	10.00				
I- H	0 / 917	-28.0	-28.0	0.54 (2)	10.00				
H- G	0 / 917	-28.0	-28.0	0.54 (2)	10.00				
G- F	0 / 0	-28.0	-28.0	0.43 (3)	10.00				



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN, C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.28")

CSI: TC=0.86/1.00 (D-E:1) , BC=0.54/1.00 (G-I:2) , WB=0.29/1.00 (D-G:1) , SSI=0.32/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (I) (INPUT = 0.90)
JSI METAL= 0.28 (I) (INPUT = 1.00)

DWG NO. TAM 5083 -B
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285814	T158	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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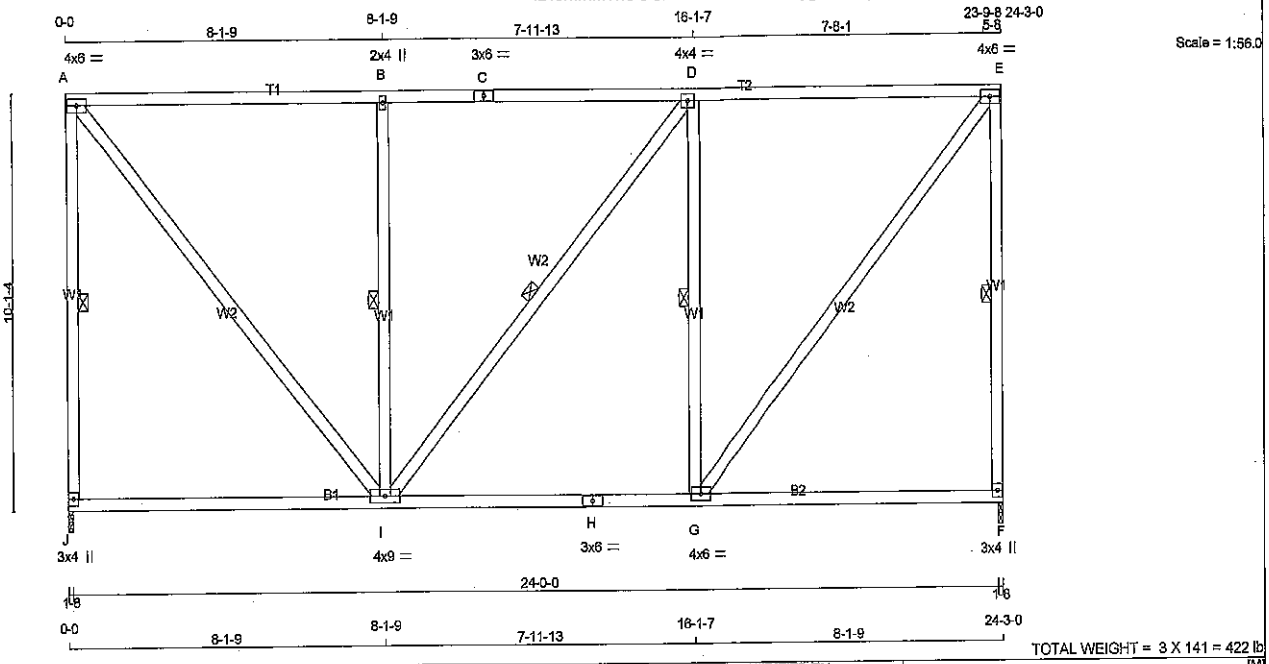

TOTAL WEIGHT = 3 X 141 = 422 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
J - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0		
B	TMW+w	MT20	2.0	4.0		
C	TS-t	MT20	3.0	6.0		
D	TMWW-t	MT20	4.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWW-t	MT20	4.0	6.0		
H	BS-t	MT20	3.0	6.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	1361	0	1361	0	0	1-8	1-8
F	1361	0	1361	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	1118	620 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
F	1118	620 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.03 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF A-J, E-F, B-I, D-I, D-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
J- A	-1269 / 0	0.0	0.0	0.63 (1)	5.72
A- B	-823 / 0	-84.3	-84.3	0.84 (1)	5.04
B- C	-823 / 0	-84.3	-84.3	0.84 (1)	5.03
C- D	-823 / 0	-84.3	-84.3	0.84 (1)	5.03
D- E	-823 / 0	-84.3	-84.3	0.85 (1)	5.03
F- E	-1269 / 0	0.0	0.0	0.63 (1)	5.71
J- I	0 / 0	-28.0	-28.0	0.43 (3)	10.00
I- H	0 / 823	-28.0	-28.0	0.53 (2)	10.00
H- G	0 / 823	-28.0	-28.0	0.53 (2)	10.00
G- F	0 / 0	-28.0	-28.0	0.43 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- I	0 / 1289	0.21 (1)
I- B	-745 / 0	0.37 (1)
I- D	-1 / 0	0.00 (1)
G- D	-746 / 0	0.37 (1)
G- E	0 / 1290	0.21 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	48.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.28")

CSI: TC=0.85/1.00 (D-E:1) , BC=0.53/1.00 (G-I:2) , WB=0.37/1.00 (D-G:1) , SSI=0.32/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.27 (I) (INPUT = 1.00)

DWG NO. TAM 50884-B
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285814	T159S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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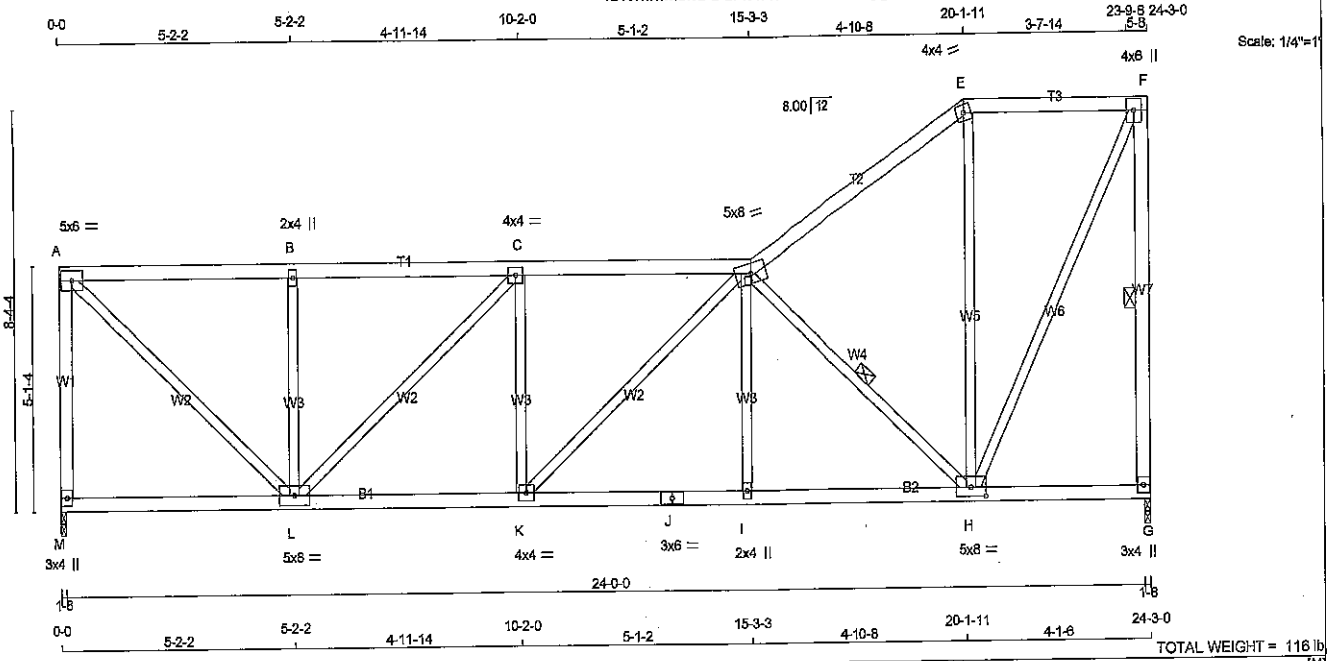


TOTAL WEIGHT = 116 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
M - A	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0		
B	TMW+w	MT20	2.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTWWW-m	MT20	5.0	8.0		
E	TTW-m	MT20	4.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	8.0	2.25	4.00
I	BMW+w	MT20	2.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMWWW-t	MT20	5.0	8.0		
M	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1361	0	1361	0	0	1-8	1-8
G	1361	0	1361	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1118	620 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
G	1118	620 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.70 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

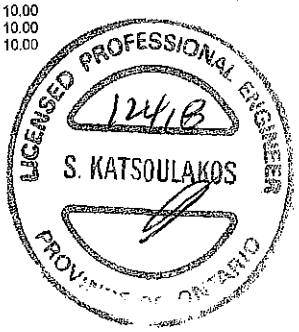
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
M- A	-1301 / 0	0.0	0.0	0.57 (1)	7.08
A- B	-1209 / 0	-84.3	-84.3	0.32 (1)	5.42
B- C	-1209 / 0	-84.3	-84.3	0.32 (1)	5.42
C- D	-1722 / 0	-84.3	-84.3	0.35 (1)	4.70
D- E	-685 / 0	-84.3	-84.3	0.36 (1)	6.25
E- F	-570 / 0	-84.3	-84.3	0.25 (1)	6.25
G- F	-1317 / 0	0.0	0.0	0.42 (1)	5.64
M- L	0 / 0	-28.0	-28.0	0.18 (3)	10.00
L- K	0 / 1722	-28.0	-28.0	0.38 (2)	10.00
K- J	0 / 1617	-28.0	-28.0	0.33 (1)	10.00
J- I	0 / 1617	-28.0	-28.0	0.33 (1)	10.00
I- H	0 / 1614	-28.0	-28.0	0.33 (1)	10.00
H- G	0 / 0	-28.0	-28.0	0.13 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- D	0 / 249	0.06 (3)
D- H	-1450 / 0	0.47 (1)
H- E	0 / 162	0.04 (3)
H- F	0 / 1280	0.29 (1)
K- D	0 / 145	0.03 (1)
A- L	0 / 1650	0.37 (1)
K- C	0 / 205	0.05 (3)
I- B	-469 / 0	0.18 (1)
L- C	-713 / 0	0.64 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.57/1.00 (A-M:1) , BC=0.38/1.00 (K-L:2) , WB=0.64/1.00 (C-L:1) , SSI=0.20/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2294	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (L) (INPUT = 0.90)
JSI METAL= 0.47 (J) (INPUT = 1.00)

DWG NO. TAM 5085 -18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285814	T160S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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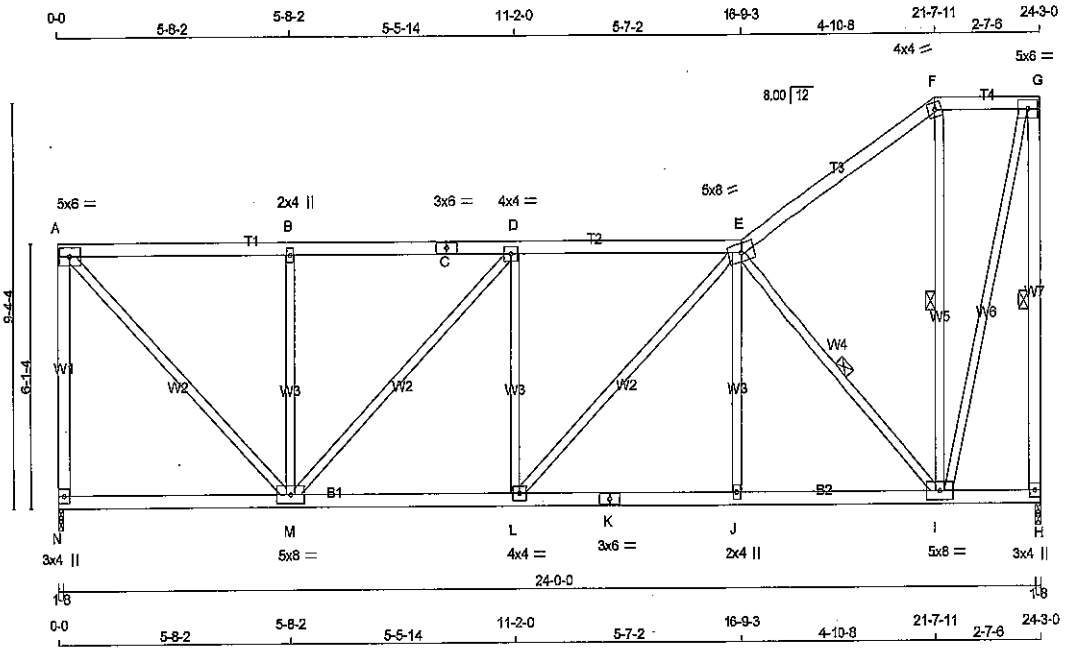

TOTAL WEIGHT = 126 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
N - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
A	TMVW-t	MT20	5.0	6.0	
B	TMW+w	MT20	2.0	4.0	
C	TS-t	MT20	3.0	6.0	
D	TMWW-t	MT20	4.0	4.0	
E	TTWWW-m	MT20	5.0	8.0	
F	TTW-m	MT20	4.0	4.0	
G	TMVW-t	MT20	5.0	6.0	
H	BMV1+p	MT20	3.0	4.0	
I	BMWWW-t	MT20	5.0	6.0	
J	BMW+w	MT20	2.0	4.0	
K	BS-t	MT20	3.0	6.0	
L	BMWW-t	MT20	4.0	4.0	
M	BMWWW-t	MT20	5.0	8.0	
N	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
N	1361	0	1361	0	0	1-8	1-8
H	1361	0	1361	0	0	1-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1118	620 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
H	1118	620 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.92 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-H, E-I, F-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO						FR-TO			
N- A	-1296 / 0	0.0	0.0	0.94 (1)	7.09	J- E	0 / 273	0.06 (2)	
A- B	-1076 / 0	-84.3	-84.3	0.39 (1)	5.55	E- I	-1358 / 0	0.53 (1)	
B- C	-1076 / 0	-84.3	-84.3	0.39 (1)	5.55	I- F	-91 / 86	0.05 (1)	
C- D	-1076 / 0	-84.3	-84.3	0.39 (1)	5.55	I- G	0 / 1271	0.29 (1)	
D- E	-1484 / 0	-84.3	-84.3	0.41 (1)	4.92	L- E	0 / 340	0.08 (1)	
E- F	-409 / 0	-84.3	-84.3	0.35 (1)	6.25	A- M	0 / 1540	0.35 (1)	
F- G	-339 / 0	-84.3	-84.3	0.10 (1)	6.25	L- D	-97 / 157	0.06 (1)	
H- G	-1333 / 0	0.0	0.0	0.55 (1)	5.61	M- B	-518 / 0	0.30 (1)	
						M- D	-565 / 0	0.75 (1)	
N- M	0 / 0	-28.0	-28.0	0.22 (3)	10.00				
M- L	0 / 1484	-28.0	-28.0	0.38 (2)	10.00				
L- K	0 / 1228	-28.0	-28.0	0.31 (2)	10.00				
K- J	0 / 1228	-28.0	-28.0	0.31 (2)	10.00				
J- I	0 / 1225	-28.0	-28.0	0.31 (2)	10.00				
I- H	0 / 0	-28.0	-28.0	0.09 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.94/1.00 (A-N:1) , BC=0.38/1.00 (L-M:2) , WB=0.75/1.00 (D-M:1) , SSI=0.22/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (M) (INPUT = 0.90)
JSI METAL= 0.40 (K) (INPUT = 1.00)

DWG NO. TAM 6292 -B

STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285814	T161S	1	1	TRUSS DESC.	

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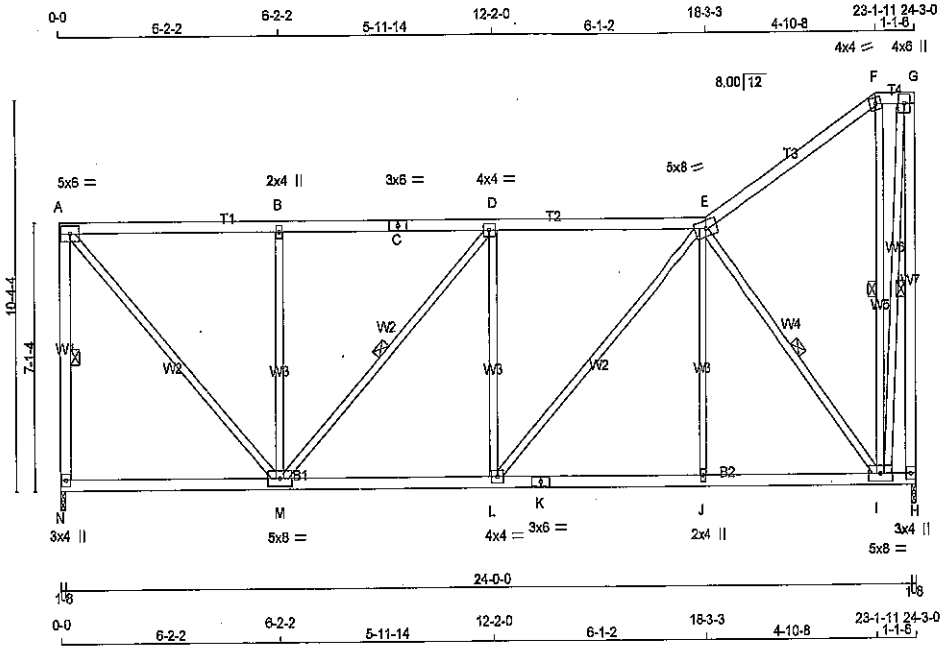


TOTAL WEIGHT = 135 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
N - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0		
B	TMW+w	MT20	2.0	4.0		
C	TS-t	MT20	3.0	6.0		
D	TMWW-t	MT20	4.0	4.0		
E	TTWWW-m	MT20	5.0	8.0		
F	TTW-m	MT20	4.0	4.0		
G	TMVW+p	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWWW-t	MT20	5.0	8.0		
J	BMW+w	MT20	2.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMWWW-t	MT20	5.0	8.0		
N	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTQRED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
N	1361	0	1361	0	0	1-8	1-8
H	1361	0	1361	0	0	1-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1118	620 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
H	1118	620 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.10 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF A-N, G-H, E-I, F-I, D-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

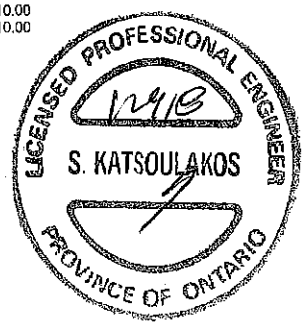
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
N- A	-1291 / 0	0.0	0.0	0.29 (1)	5.66	
A- B	-979 / 0	-84.3	-84.3	0.46 (1)	5.83	
B- C	-979 / 0	-84.3	-84.3	0.46 (1)	5.63	
C- D	-979 / 0	-84.3	-84.3	0.46 (1)	5.63	
D- E	-1285 / 0	-84.3	-84.3	0.48 (1)	5.10	
E- F	-152 / 0	-84.3	-84.3	0.34 (1)	6.25	
F- G	-124 / 0	-84.3	-84.3	0.02 (1)	6.25	
H- G	-1287 / 0	0.0	0.0	0.68 (1)	5.69	
N- M	0 / 0	-28.0	-28.0	0.26 (3)	10.00	
M- L	0 / 1286	-28.0	-28.0	0.40 (2)	10.00	
L- K	0 / 918	-28.0	-28.0	0.31 (2)	10.00	
K- J	0 / 918	-28.0	-28.0	0.31 (2)	10.00	
J- I	0 / 915	-28.0	-28.0	0.31 (2)	10.00	
I- H	0 / 0	-28.0	-28.0	0.08 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- E	0 / 304	0.07 (2)
E- I	-1336 / 0	0.64 (1)
I- F	-170 / 22	0.13 (1)
I- G	0 / 1248	0.28 (1)
L- E	0 / 520	0.12 (1)
A- M	0 / 1458	0.33 (1)
L- D	-230 / 109	0.20 (1)
M- B	-553 / 0	0.49 (1)
M- D	-433 / 0	0.25 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.68/1.00 (G-H:1) , BC=0.40/1.00 (L-M:2) , WB=0.64/1.00 (E-I:1) , SSI=0.24/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (E) (INPUT = 0.90)
JSI METAL= 0.28 (I) (INPUT = 1.00)

DWG NO. TAM 6293
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285812	T162	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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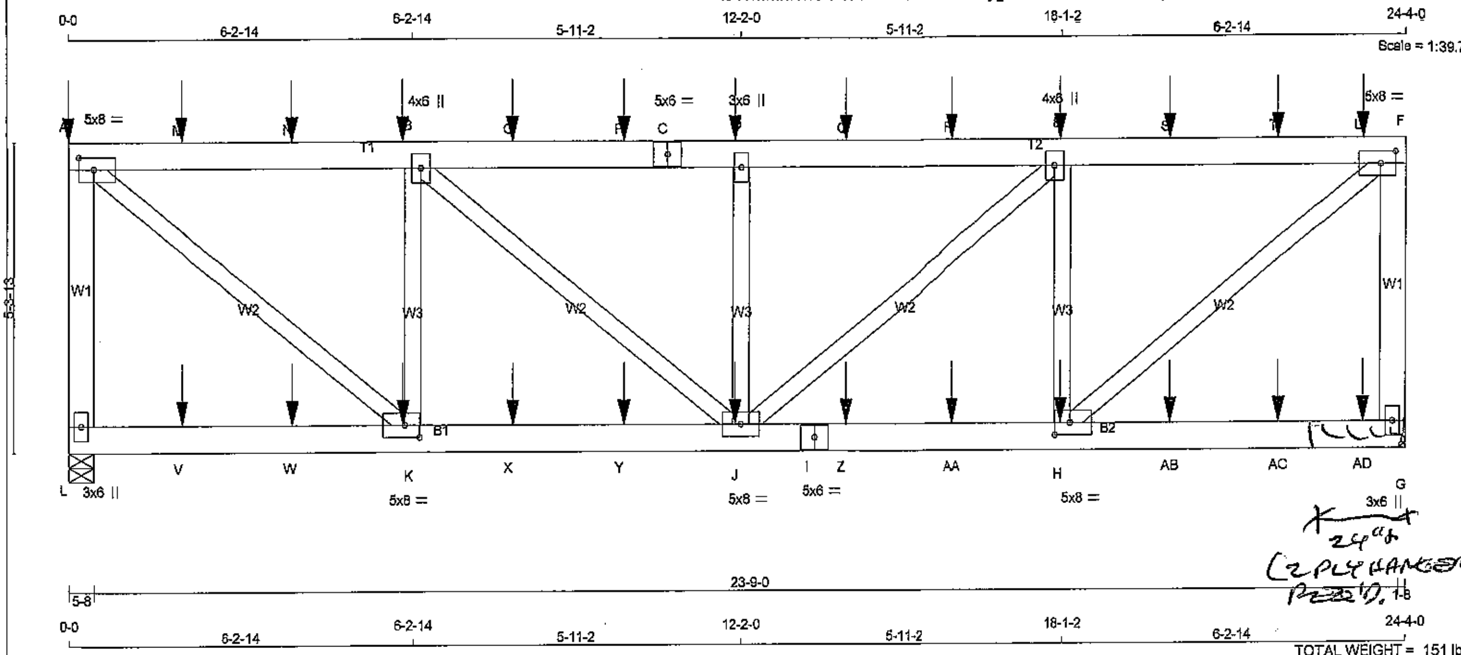

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
L - A	2x6	DRY	No.2	SPF
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
G - F	2x6	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
I - G	2x6	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	8.0	2.50	3.25
B	TMWW+t	MT20	4.0	6.0		
C	TS-t	MT20	5.0	6.0		
D	TMW+w	MT20	3.0	6.0		
E	TMWW+t	MT20	4.0	6.0		
F	TMVW-t	MT20	5.0	8.0	2.50	3.25
G	BMV1+p	MT20	3.0	6.0		
H	BMWW-t	MT20	5.0	8.0	2.50	3.25
I	BS-t	MT20	5.0	6.0		
J	BMWWW-t	MT20	5.0	8.0		
K	BMWW-t	MT20	5.0	8.0	2.50	3.25
L	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 140.9 lbs FACTORED DOWN AT 0-0, 101.4 lbs FACTORED DOWN AT 2-0-12, 101.4 lbs FACTORED DOWN AT 4-0-12, 101.4 lbs FACTORED DOWN AT 6-0-12, 101.4 lbs FACTORED DOWN AT 8-0-12, 101.4 lbs FACTORED DOWN AT 10-0-12, 101.4 lbs FACTORED DOWN AT 12-0-12, 101.4 lbs FACTORED DOWN AT 14-0-12, 152.7 lbs FACTORED DOWN AT 16-0-12, 152.7 lbs FACTORED DOWN AT 18-0-12, 152.7 lbs FACTORED DOWN AT 20-0-12, AND 152.7 lbs FACTORED DOWN AT 22-0-12, AND 162.4 lbs FACTORED DOWN AT 23-6-12 ON TOP CHORD, AND 71.7 lbs FACTORED DOWN AT 2-0-12, 71.7 lbs FACTORED DOWN AT 4-0-12, 71.7 lbs FACTORED DOWN AT 6-0-12, 71.7 lbs FACTORED DOWN AT 8-0-12, 71.7 lbs FACTORED DOWN AT 10-0-12, 71.7 lbs FACTORED DOWN AT 12-0-12, 71.7 lbs FACTORED DOWN AT 14-0-12, 88.8 lbs FACTORED DOWN AT 16-0-12, 88.8 lbs FACTORED DOWN AT 18-0-12, 88.8 lbs FACTORED DOWN AT 20-0-12, AND 88.8 lbs FACTORED DOWN AT 22-0-12, AND 94.4 lbs FACTORED DOWN AT 23-6-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

SCAB JOINT # G WITH 2 X 6 #2 SPF 24" LONG 1 SIDE(S) USING 2 ROW(S) 3 1/4" COMMON WIRE NAILS AT 2" o/c STAGGERED (20 NAILS/SCAB) REQUIRED FOR 2" BRG PROVISION.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	2354	0	2354	0	0	5-8	5-8
G	2545	0	2545	0	0	HANGER BY OTHERS	MIN. SEAT SIZE: 2-12

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1928	1079 / 0	433 / 0	0 / 0	0 / 0	415 / 0	0 / 0
G	2097	1152 / 0	486 / 0	0 / 0	0 / 0	459 / 0	0 / 0

(MAX./MIN. COMPONENT REACTIONS)

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.24 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
L- A	-2249 / 0	0.0	0.0	0.61 (1)	6.79
A- M	-2334 / 0	-84.3	-84.3	0.41 (1)	4.88
M- N	-2334 / 0	-84.3	-84.3	0.41 (1)	4.88
N- B	-2334 / 0	-84.3	-84.3	0.41 (1)	4.88
B- O	-3010 / 0	-84.3	-84.3	0.43 (1)	4.37
O- P	-3010 / 0	-84.3	-84.3	0.43 (1)	4.37
P- C	-3010 / 0	-84.3	-84.3	0.43 (1)	4.37
C- D	-3010 / 0	-84.3	-84.3	0.43 (1)	4.37
D- Q	-3010 / 0	-84.3	-84.3	0.52 (1)	4.24
Q- R	-3010 / 0	-84.3	-84.3	0.52 (1)	4.24
R- E	-3010 / 0	-84.3	-84.3	0.52 (1)	4.24
E- S	-2480 / 0	-84.3	-84.3	0.49 (1)	4.62
S- T	-2480 / 0	-84.3	-84.3	0.49 (1)	4.62
T- U	-2480 / 0	-84.3	-84.3	0.49 (1)	4.62
U- F	-2480 / 0	-84.3	-84.3	0.49 (1)	4.62
G- F	-2389 / 0	0.0	0.0	0.65 (1)	6.62
L- V	0 / 0	-28.0	-28.0	0.22 (3)	10.00
V- W	0 / 0	-28.0	-28.0	0.22 (3)	10.00
W- K	0 / 0	-28.0	-28.0	0.22 (3)	10.00
K- X	0 / 2334	-28.0	-28.0	0.44 (2)	10.00
X- Y	0 / 2334	-28.0	-28.0	0.44 (2)	10.00
Y- J	0 / 2334	-28.0	-28.0	0.44 (2)	10.00
J- I	0 / 2480	-28.0	-28.0	0.49 (2)	10.00
I- Z	0 / 2480	-28.0	-28.0	0.49 (2)	10.00
Z-AA	0 / 2480	-28.0	-28.0	0.49 (2)	10.00
AA- H	0 / 2480	-28.0	-28.0	0.49 (2)	10.00
H-AB	0 / 0	-28.0	-28.0	0.25 (3)	10.00
AB-AC	0 / 0	-28.0	-28.0	0.25 (3)	10.00
AC-AD	0 / 0	-28.0	-28.0	0.25 (3)	10.00
AD- G	0 / 0	-28.0	-28.0	0.25 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- F	0 / 3143	0.56 (1)
A- K	0 / 2958	0.52 (1)
H- E	-1559 / 0	0.42 (1)
K- B	-1485 / 0	0.40 (1)
J- E	0 / 685	0.12 (1)
B- J	0 / 874	0.15 (1)
J- D	-730 / 0	0.20 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
A	0-0	-141	-141	---	BACK	VERT	TOTAL
B	6-0-12	-101	-101	—	BACK	VERT	TOTAL
D	12-0-12	-101	-101	---	BACK	VERT	TOTAL
E	18-0-12	-153	-153	---	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.65 (F-G:1) , BC=0.49 (H-J:2) , WB=0.56 (F-H:1) , SSI=0.36 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.46 (F) (INPUT = 1.00)

p6 1/2

DWG NO. TAM49994-17
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
H	18-0-12	-51	-89	—	BACK	VERT	TOTAL
J	12-0-12	-41	-72	—	BACK	VERT	TOTAL
K	6-0-12	-41	-72	—	BACK	VERT	TOTAL
M	2-0-12	-101	-101	—	BACK	VERT	TOTAL
N	4-0-12	-101	-101	—	BACK	VERT	TOTAL
O	8-0-12	-101	-101	—	BACK	VERT	TOTAL
P	10-0-12	-101	-101	—	BACK	VERT	TOTAL
Q	14-0-12	-101	-101	—	BACK	VERT	TOTAL
R	16-0-12	-153	-153	—	BACK	VERT	TOTAL
S	20-0-12	-153	-153	—	BACK	VERT	TOTAL
T	22-0-12	-153	-153	—	BACK	VERT	TOTAL
U	23-6-12	-162	-162	—	BACK	VERT	TOTAL
V	2-0-12	-41	-72	—	BACK	VERT	TOTAL
W	4-0-12	-41	-72	—	BACK	VERT	TOTAL
X	8-0-12	-41	-72	—	BACK	VERT	TOTAL
Y	10-0-12	-41	-72	—	BACK	VERT	TOTAL
Z	14-0-12	-41	-72	—	BACK	VERT	TOTAL
AA	16-0-12	-51	-89	—	BACK	VERT	TOTAL
AB	20-0-12	-51	-89	—	BACK	VERT	TOTAL
AC	22-0-12	-51	-89	—	BACK	VERT	TOTAL
AD	23-6-12	-54	-94	—	BACK	VERT	TOTAL

DWG NO. TAM 99994-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285812	T163	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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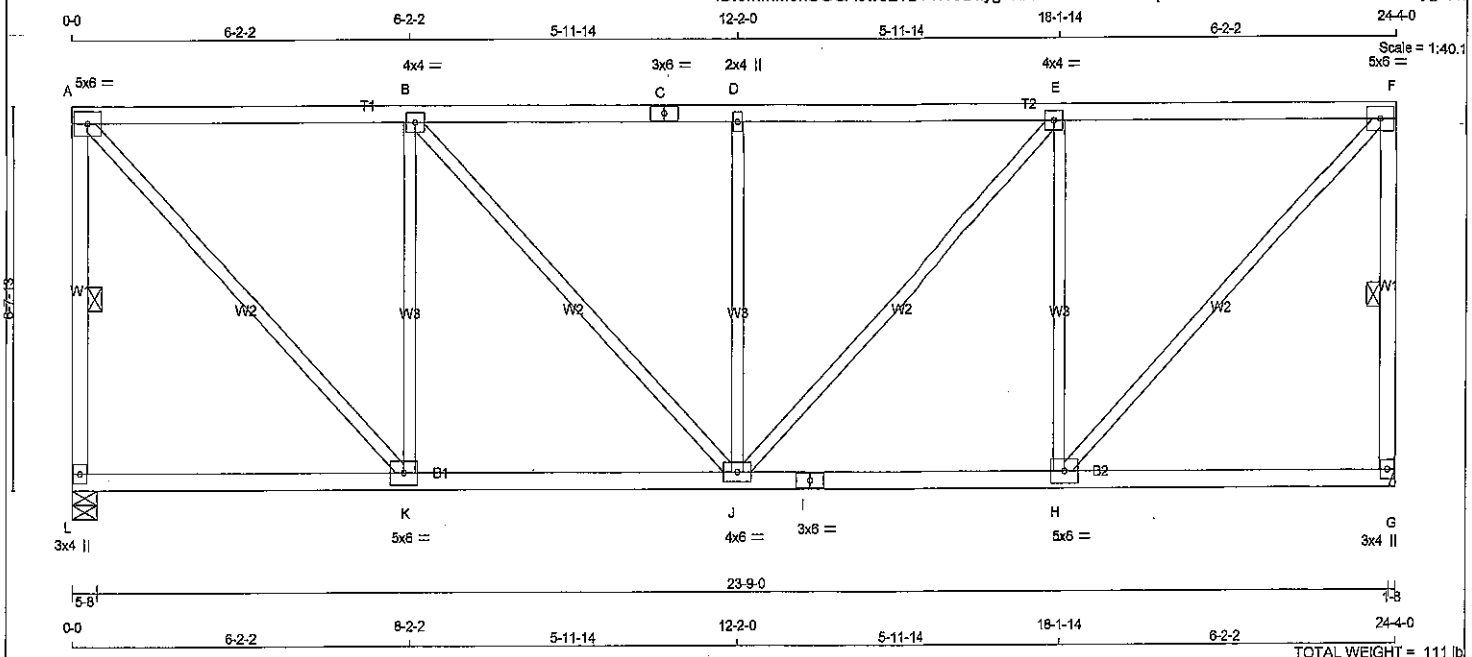


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
L - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0		
B	TMWW-t	MT20	4.0	4.0		
C	TS-t	MT20	3.0	6.0		
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMVW-t	MT20	5.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	5.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
L	1366	0	1366	0	0	5-8	5-8
G	1366	0	1366	0	0	HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1121	623 / 0	256 / 0	0 / 0	0 / 0	243 / 0	0 / 0
G	1121	623 / 0	256 / 0	0 / 0	0 / 0	243 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.68 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

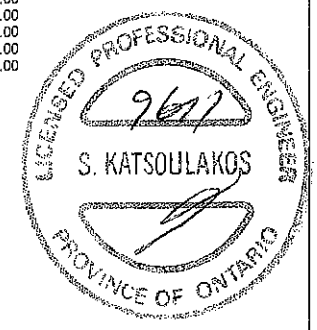
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF A-L, F-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- A	-1296 / 0	0.0	0.0	0.26 (1)	5.67	H- F	0 / 1519	0.34 (1)
A- B	-1058 / 0	-84.3	-84.3	0.51 (1)	5.38	A- K	0 / 1519	0.34 (1)
B- C	-1347 / 0	-84.3	-84.3	0.53 (1)	4.88	H- E	-894 / 0	0.65 (1)
C- D	-1347 / 0	-84.3	-84.3	0.53 (1)	4.88	K- B	-894 / 0	0.65 (1)
D- E	-1347 / 0	-84.3	-84.3	0.53 (1)	4.88	J- E	0 / 421	0.09 (1)
E- F	-1058 / 0	-84.3	-84.3	0.51 (1)	5.38	B- J	0 / 421	0.09 (1)
G- F	-1296 / 0	0.0	0.0	0.26 (1)	5.67	J- D	-462 / 0	0.34 (1)
L- K	0 / 0	-28.0	-28.0	0.26 (3)	10.00			
K- J	0 / 1058	-28.0	-28.0	0.39 (2)	10.00			
J- I	0 / 1058	-28.0	-28.0	0.39 (2)	10.00			
I- H	0 / 1058	-28.0	-28.0	0.39 (2)	10.00			
H- G	0 / 0	-28.0	-28.0	0.26 (3)	10.00			



DWG NO. TAM 49995-17

STRUCTURAL COMPONENT ONLY

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.53 (D-E:1) , BC=0.39 (J-K:2) , WB=0.65 (E-H:1) , SSI=0.24 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (H) (INPUT = 0.90)
JSI METAL= 0.30 (I) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285812	T164	11	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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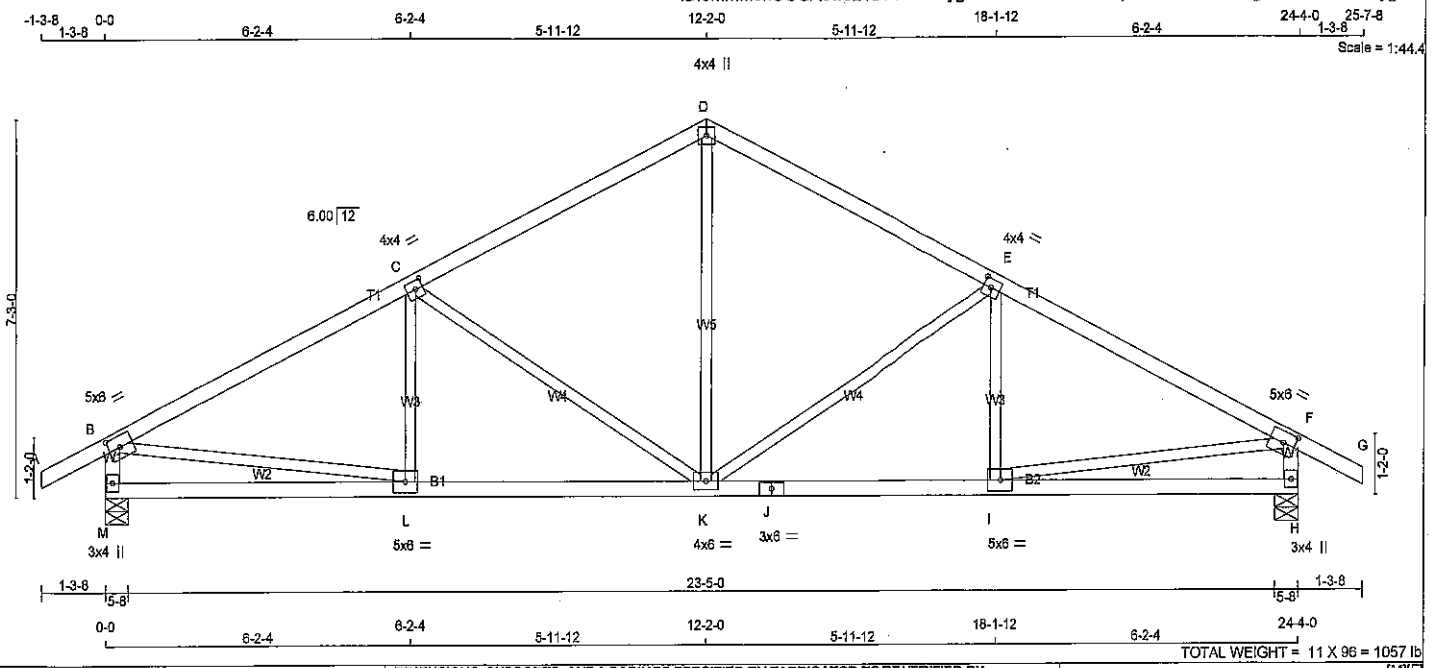

TOTAL WEIGHT = 11 X 96 = 1057 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTW+p	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.75
F	TMVW-t	MT20	5.0	6.0	2.50	2.75
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
M	1480	0	1480	0	0	5-8	5-8
H	1480	0	1480	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1199	692 / 0	256 / 0	0 / 0	0 / 0	251 / 0	0 / 0
H	1199	692 / 0	256 / 0	0 / 0	0 / 0	251 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.40 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)		LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO			FR-TO		
A- B	0 / 26	-84.3	-84.3	0.11 (1)	10.00	K- D	0 / 828	0.19 (1)
B- C	-1886 / 0	-84.3	-84.3	0.47 (1)	4.40	K- E	-603 / 0	0.60 (1)
C- D	-1378 / 0	-84.3	-84.3	0.43 (1)	5.04	I- E	-50 / 219	0.05 (3)
D- E	-1378 / 0	-84.3	-84.3	0.43 (1)	5.04	C- K	-603 / 0	0.60 (1)
E- F	-1886 / 0	-84.3	-84.3	0.47 (1)	4.40	L- C	-50 / 219	0.05 (3)
F- G	0 / 26	-84.3	-84.3	0.11 (1)	10.00	B- L	0 / 1727	0.39 (1)
M- B	-1408 / 0	0.0	0.0	0.14 (1)	6.87	I- F	0 / 1727	0.39 (1)
H- F	-1408 / 0	0.0	0.0	0.14 (1)	6.87			
M- L	0 / 0	-28.0	-28.0	0.25 (3)	10.00			
L- K	0 / 1711	-28.0	-28.0	0.45 (2)	10.00			
K- J	0 / 1711	-28.0	-28.0	0.45 (2)	10.00			
J- I	0 / 1711	-28.0	-28.0	0.45 (2)	10.00			
I- H	0 / 0	-28.0	-28.0	0.25 (3)	10.00			

DWG NO. TAM49996-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.47 (B-C:1) , BC=0.45 (K-L:2) , WB=0.60 (C-K:1) , SSI=0.22 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (L) (INPUT = 0.90)
JSI METAL= 0.51 (F) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285812	T165A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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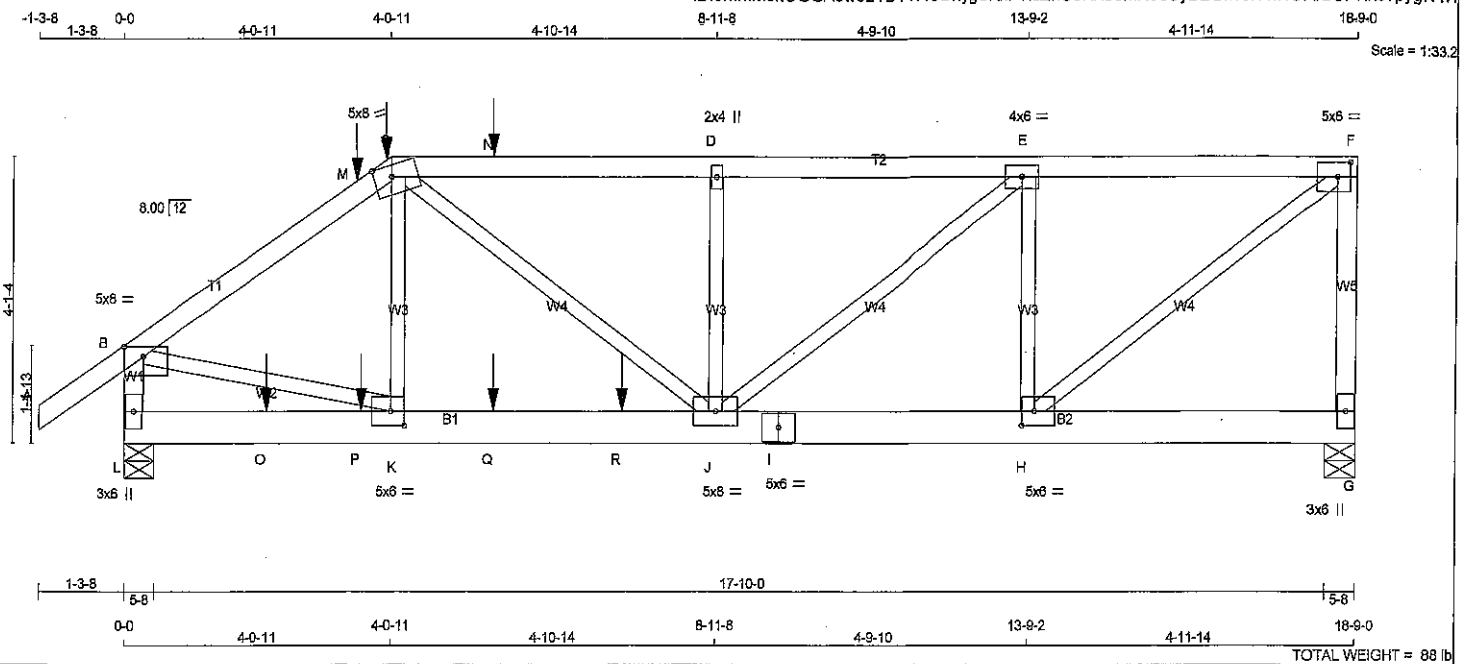

TOTAL WEIGHT = 88 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
I - G	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.75	3.50
C	TTWW-m	MT20	5.0	8.0	2.00	3.25
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	8.0		
F	TMVW-t	MT20	5.0	8.0	2.50	2.25
G	BMV1+p	MT20	3.0	6.0		
H	BMWW-t	MT20	5.0	8.0	2.50	2.25
I	BS-t	MT20	5.0	6.0		
J	BMWWW-t	MT20	5.0	8.0		
K	BMWW-t	MT20	5.0	6.0	2.50	2.50
L	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 244.5 lbs FACTORED DOWN AT 4-0-11, AND 124.6 lbs FACTORED DOWN AT 3-7-4, AND 101.4 lbs FACTORED DOWN AT 5-7-4 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 2-2-0, 69.9 lbs FACTORED DOWN AT 3-7-4, AND 69.9 lbs FACTORED DOWN AT 5-7-4, AND 1040.6 lbs FACTORED DOWN AT 7-8-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DWG NO. TAM49997-17

STRUCTURAL

COMPONENT ONLY

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1602	0	1602	0	0	5-8	5-8
L	2249	0	2249	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1294	753 / 0	272 / 0	0 / 0	0 / 0	270 / 0	0 / 0
L	1795	1080 / 0	353 / 0	0 / 0	0 / 0	362 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.44 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 32	-84.3	-84.3	0.12 (1)	10.00	K- C	0 / 324	0.08 (3)
B- M	-2543 / 0	-84.3	-84.3	0.45 (1)	3.85	B- K	0 / 2195	0.54 (1)
M- C	-2543 / 0	-84.3	-84.3	0.45 (1)	3.85	H- F	0 / 2254	0.56 (1)
C- N	-2908 / 0	-84.3	-84.3	0.59 (1)	3.44	C- J	0 / 966	0.24 (1)
N- D	-2908 / 0	-84.3	-84.3	0.59 (1)	3.44	H- E	-1303 / 0	0.33 (1)
D- E	-2908 / 0	-84.3	-84.3	0.49 (1)	3.59	J- D	-477 / 0	0.12 (1)
E- F	-1806 / 0	-84.3	-84.3	0.36 (1)	4.57	J- E	0 / 1396	0.35 (1)
G- F	-1522 / 0	0.0	0.0	0.38 (1)	6.62			
L- B	-2243 / 0	0.0	0.0	0.25 (1)	5.63			
L- O	0 / 0	-28.0	-28.0	0.23 (1)	10.00			
O- P	0 / 0	-28.0	-28.0	0.23 (1)	10.00			
P- K	0 / 0	-28.0	-28.0	0.23 (1)	10.00			
K- Q	0 / 2137	-28.0	-28.0	0.86 (1)	10.00			
Q- R	0 / 2137	-28.0	-28.0	0.86 (1)	10.00			
R- J	0 / 2137	-28.0	-28.0	0.86 (1)	10.00			
J- I	0 / 1806	-28.0	-28.0	0.48 (1)	10.00			
I- H	0 / 1806	-28.0	-28.0	0.48 (1)	10.00			
H- G	0 / 0	-28.0	-28.0	0.10 (2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	4-0-11	-23	-25	—	FRONT	VERT	DEAD
C	4-0-11	-222	-222	—	FRONT	VERT	SNOW
M	3-7-4	-125	-125	—	FRONT	VERT	TOTAL
N	5-7-4	-101	-101	—	FRONT	VERT	TOTAL
O	2-2-0	-40	-70	—	FRONT	VERT	TOTAL
P	3-7-4	-40	-70	—	FRONT	VERT	TOTAL
Q	5-7-4	-40	-70	—	FRONT	VERT	TOTAL
R	7-6-8	-1041	-1041	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.59 (C-D:1) , BC=0.86 (J-K:1) , WB=0.56 (F-H:1) , SSI=0.54 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (K) (INPUT = 0.90)
JSI METAL= 0.53 (K) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285812	T166A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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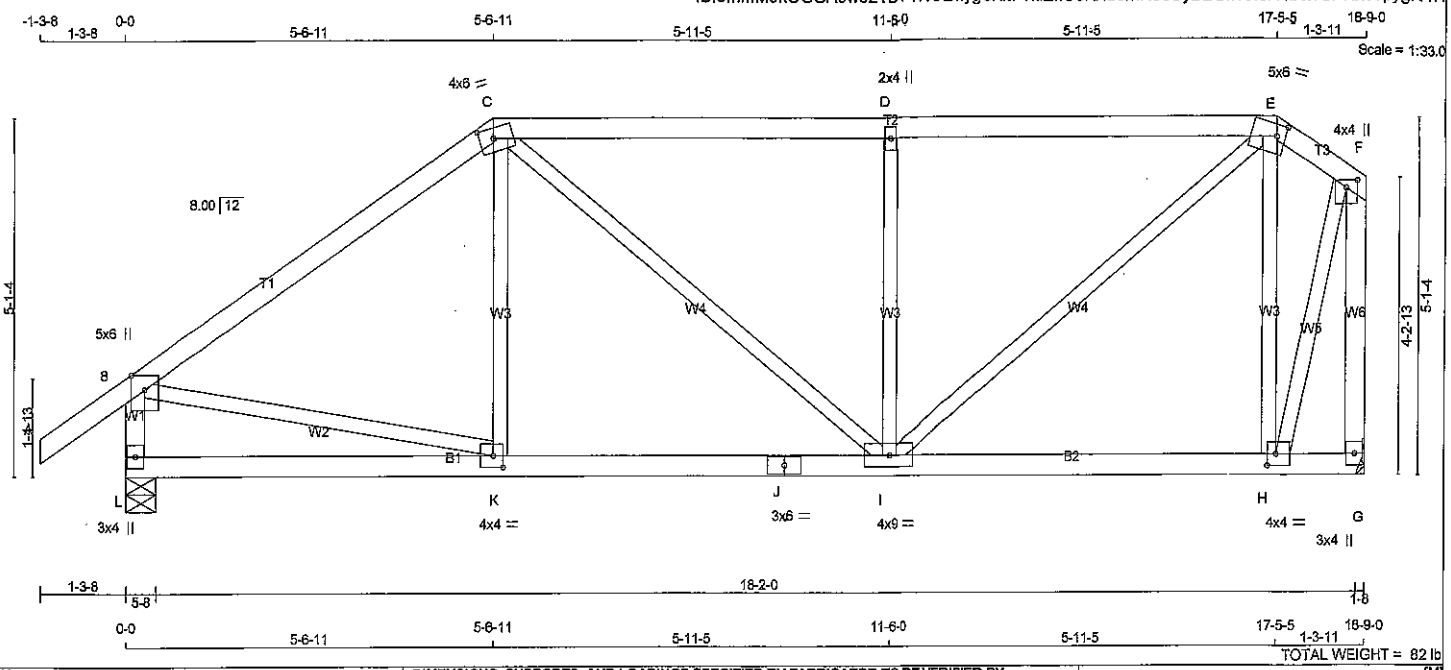

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW-m	MT20	4.0	6.0	1.75	2.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW-m	MT20	5.0	6.0	2.00	1.50
F	TMVW+p	MT20	4.0	4.0	1.25	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	4.0	4.0	2.00	1.50
I	BMWWW-t	MT20	4.0	9.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0	2.00	1.75
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT	REQRD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	BRG
L	1188	0	1188	0	0	IN-SX	IN-SX
G	1052	0	1052	0	0	5-8	5-8
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	943	550 / 0	197 / 0	0 / 0	0 / 0	196 / 0	0 / 0
G	864	480 / 0	197 / 0	0 / 0	0 / 0	186 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.30 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)		LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO				FR-TO		
A- B	0 / 32	-84.3	-84.3	0.11 (1)		10.00	K- C	0 / 227	0.05 (3)
B- C	-1057 / 0	-84.3	-84.3	0.51 (1)		5.41	C- I	0 / 235	0.05 (1)
C- D	-1061 / 0	-84.3	-84.3	0.54 (1)		5.30	I- D	-618 / 0	0.24 (1)
D- E	-1061 / 0	-84.3	-84.3	0.54 (1)		5.30	I- E	0 / 1002	0.23 (1)
E- F	-358 / 0	-84.3	-84.3	0.03 (1)		6.25	H- E	-676 / 0	0.26 (1)
L- B	-1105 / 0	0.0	0.0	0.11 (1)		7.53	B- K	0 / 894	0.20 (1)
G- F	-1078 / 0	0.0	0.0	0.32 (1)		7.60	H- F	0 / 874	0.20 (1)
L- K	0 / 0	-28.0	-28.0	0.22 (3)		10.00			
K- J	0 / 878	-28.0	-28.0	0.33 (2)		10.00			
J- I	0 / 878	-28.0	-28.0	0.33 (2)		10.00			
I- H	0 / 281	-28.0	-28.0	0.22 (2)		10.00			
H- G	0 / 0	-28.0	-28.0	0.15 (2)		10.00			

DWG NO. TAM4999B17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.54 (D-E:1) , BC=0.33 (I-K:2) , WB=0.26 (E-H:1) , SSI=0.24 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches .

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (C) (INPUT = 0.90)
JSI METAL= 0.33 (K) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285812	T167A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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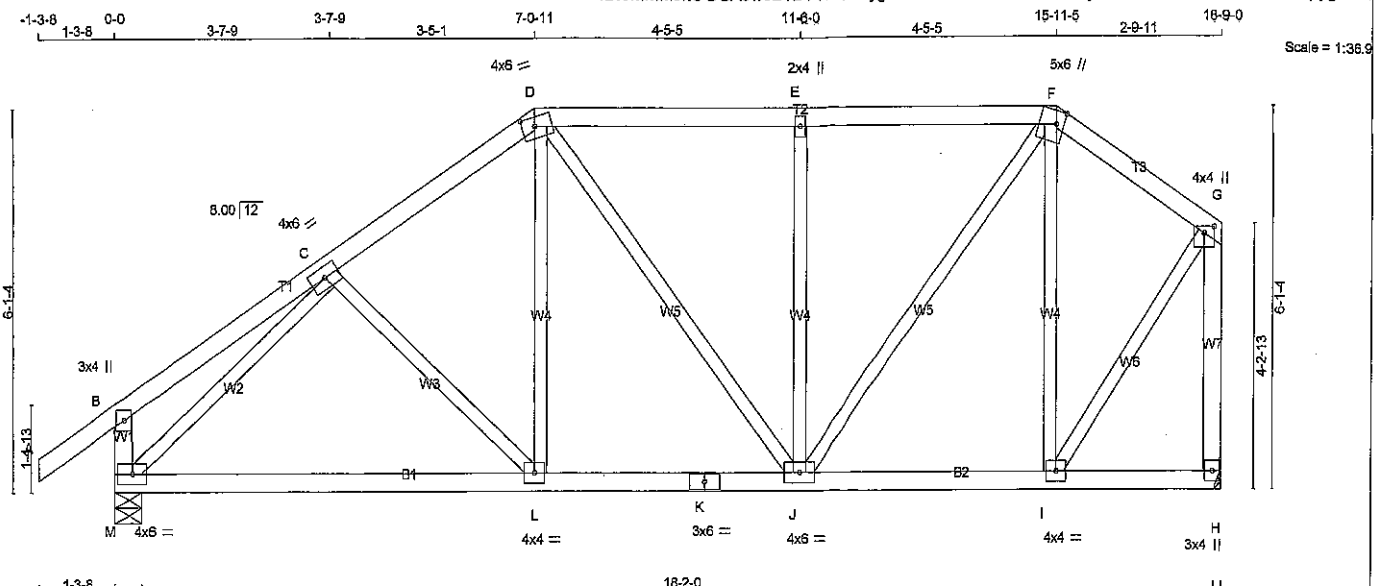


TOTAL WEIGHT = 89 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW-m	MT20	4.0	6.0	1.75	2.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.50	1.50
G	TMVW+p	MT20	4.0	4.0	1.25	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BS-t	MT20	3.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
M	1168	0	1168	0	0	5-8	5-8
H	1052	0	1052	0	0	HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	943	550 / 0	197 / 0	0 / 0	0 / 0	196 / 0	0 / 0
H	864	480 / 0	197 / 0	0 / 0	0 / 0	188 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

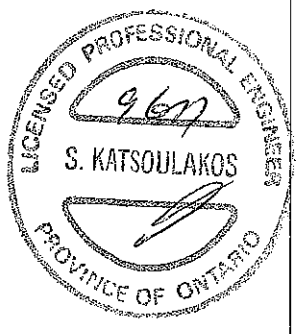
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.10 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	C- L	-82 / 75	0.03 (1)
B- C	0 / 18	-84.3	-84.3	0.16 (1)	10.00	L- D	0 / 333	0.08 (2)
C- D	-1002 / 0	-84.3	-84.3	0.13 (1)	6.10	D- J	-49 / 32	0.05 (3)
D- E	-841 / 0	-84.3	-84.3	0.21 (1)	6.25	J- E	-457 / 0	0.27 (1)
E- F	-841 / 0	-84.3	-84.3	0.21 (1)	6.25	J- F	0 / 656	0.15 (1)
F- G	-543 / 0	-84.3	-84.3	0.09 (1)	6.25	I- F	-482 / 0	0.28 (1)
M- B	-231 / 0	0.0	0.0	0.02 (1)	7.81	M- C	-1224 / 0	0.47 (1)
H- G	-1028 / 0	0.0	0.0	0.31 (1)	7.74	I- G	0 / 756	0.17 (1)
M- L	0 / 877	-28.0	-28.0	0.40 (2)	10.00			
L- K	0 / 822	-28.0	-28.0	0.41 (2)	10.00			
K- J	0 / 822	-28.0	-28.0	0.41 (2)	10.00			
J- I	0 / 442	-28.0	-28.0	0.18 (2)	10.00			
I- H	0 / 0	-28.0	-28.0	0.11 (2)	10.00			



DWG NO. TAM49999-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.31 (G-H:1) , BC=0.41 (J-L:2) , WB=0.47 (C-M:1) , SSI=0.18 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (J) (INPUT = 0.90)
JSI METAL= 0.30 (C) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285812	T168A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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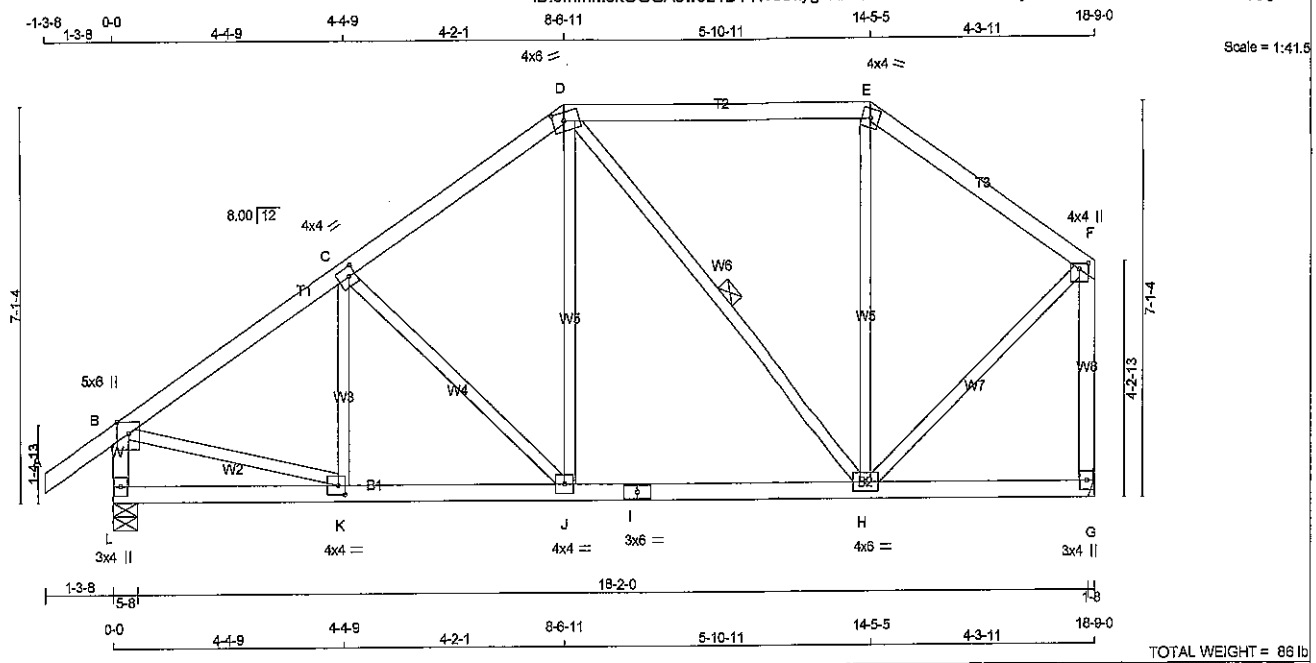


TOTAL WEIGHT = 86 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	4.0	6.0	1.75	2.50
E	TTW-m	MT20	4.0	4.0		
F	TMVW+p	MT20	4.0	4.0	1.25	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	4.0	2.00	1.50
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
L	1168	0	1168	0	0	5-8	5-8
G	1052	0	1052	0	0	HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	943	550 / 0	197 / 0	0 / 0	0 / 0	196 / 0	0 / 0
G	864	460 / 0	197 / 0	0 / 0	0 / 0	188 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.79 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

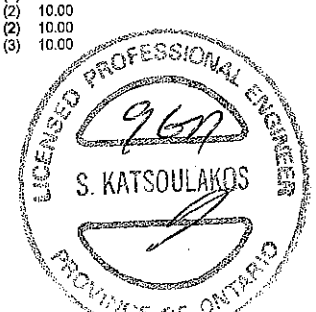
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	K- C	-105 / 111	0.03 (1)	
B- C	-1105 / 0	-84.3	-84.3	0.21 (1)	5.79	C- J	-293 / 0	0.17 (1)	
C- D	-896 / 0	-84.3	-84.3	0.20 (1)	6.25	J- D	0 / 379	0.09 (2)	
D- E	-529 / 0	-84.3	-84.3	0.38 (1)	6.25	D- H	-309 / 0	0.17 (1)	
E- F	-638 / 0	-84.3	-84.3	0.21 (1)	6.25	H- E	-74 / 149	0.06 (1)	
L- B	-1116 / 0	0.0	0.0	0.11 (1)	7.51	B- K	0 / 956	0.22 (1)	
G- F	-1007 / 0	0.0	0.0	0.30 (1)	7.80	H- F	0 / 709	0.16 (1)	
L- K	0 / 0	-28.0	-28.0	0.11 (3)	10.00				
K- J	0 / 938	-28.0	-28.0	0.25 (2)	10.00				
J- I	0 / 730	-28.0	-28.0	0.27 (2)	10.00				
I- H	0 / 730	-28.0	-28.0	0.27 (2)	10.00				
H- G	0 / 0	-28.0	-28.0	0.18 (3)	10.00				



DWG NO. TAM50000-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.38 (D-E:1) , BC=0.27 (H-J:2) , WB=0.22 (B-K:1) , SSI=0.19 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (H) (INPUT = 0.90)
JSI METAL= 0.35 (K) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285812	T169A	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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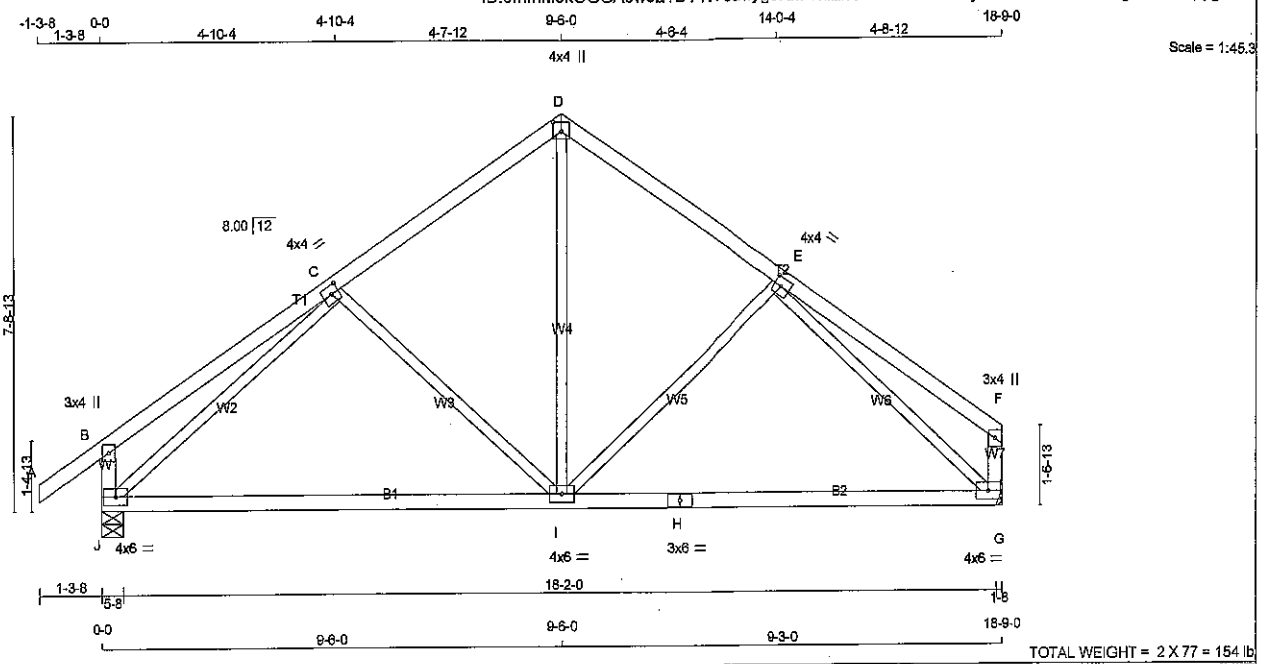

TOTAL WEIGHT = 2 X 77 = 154 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTW+p	MT20	4.0	4.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0	2.00	1.75
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	6.0		
H	BS-t	MT20	3.0	6.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT	REQRD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	BRG
J	1188	0	1188	0	0	IN-SX	IN-SX
G	1052	0	1052	0	0	5-8	5-8
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	943	550 / 0	197 / 0	0 / 0	0 / 0	196 / 0	0 / 0
G	864	460 / 0	197 / 0	0 / 0	0 / 0	168 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	C- I	-250 / 38	0.18 (1)
B- C	0 / 26	-84.3	-84.3	0.31 (1)	10.00	I- D	0 / 637	0.14 (1)
C- D	-874 / 0	-84.3	-84.3	0.25 (1)	6.25	I- E	-212 / 51	0.15 (1)
D- E	-873 / 0	-84.3	-84.3	0.23 (1)	6.25	J- C	-1197 / 0	0.85 (1)
E- F	0 / 25	-84.3	-84.3	0.29 (1)	10.00	E- G	-1179 / 0	0.82 (1)
J- B	-268 / 0	0.0	0.0	0.03 (1)	7.81			
G- F	-149 / 0	0.0	0.0	0.02 (1)	7.81			
J- I	0 / 892	-28.0	-28.0	0.82 (2)	10.00			
I- H	0 / 861	-28.0	-28.0	0.82 (2)	10.00			
H- G	0 / 861	-28.0	-28.0	0.82 (2)	10.00			

DWG NO. TAM5000H17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	48.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.83")
CALCULATED VERT. DEFL.(LL) = L/ 937 (0.24")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 564 (0.40")

CSI: TC=0.31 (B-C:1) , BC=0.82 (I-J:2) , WB=0.85 (C-J:1) , SSI=0.23 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(ORY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (D) (INPUT = 0.90)
JSI METAL= 0.42 (C) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43854	DRWG NO.
285812	T170A	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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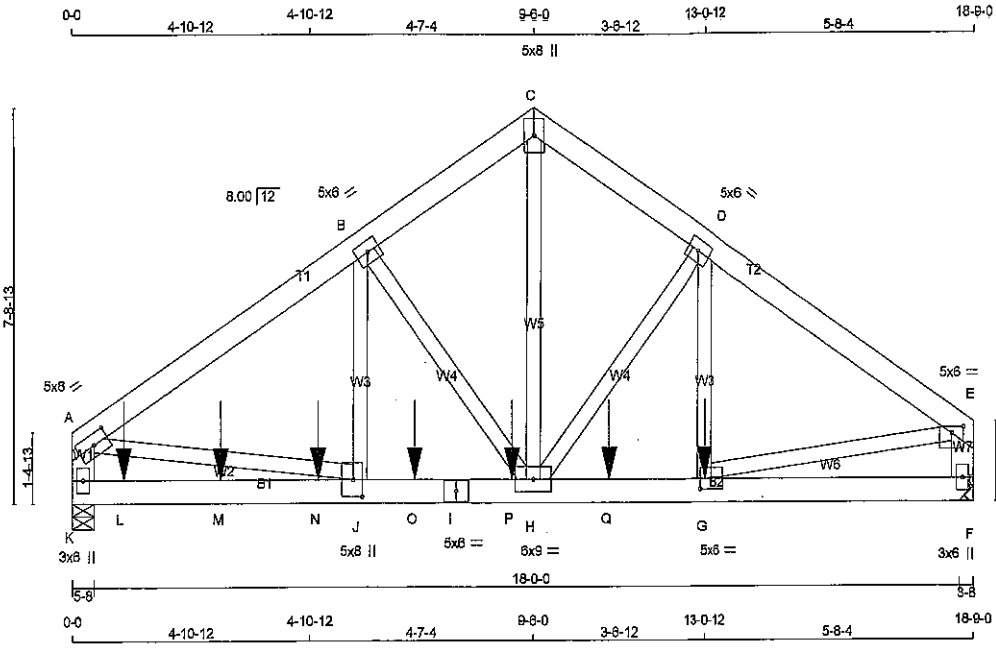


TOTAL WEIGHT = 3 X 120 = 361 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
K - A	2x6	DRY	No.2	SPF
F - E	2x6	DRY	No.2	SPF
K - I	2x6	DRY	2100F 1.8E	SPF
I - F	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	TOP
C- E	2	12	TOP
K- A	2	12	TOP
F- E	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
K- I	2	10	SIDE(378.6)
I- F	2	11	SIDE(395.6)
WEBS : (0.122"X3") SPIRAL NAILS			
D- G	1	5	SIDE(348.1)
2x4	1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	8.0	2.50	3.75
B	TMWW-t	MT20	5.0	6.0		
C	TTW+p	MT20	5.0	8.0		
D	TMWW-t	MT20	5.0	6.0		
E	TMVW-p	MT20	5.0	6.0	1.50	3.00
F	BMV1+p	MT20	3.0	6.0		
G	BMWW-t	MT20	5.0	6.0	2.50	2.75
H	BMWWW-t	MT20	6.0	9.0		
I	BS-t	MT20	5.0	6.0		
J	BMWW+t	MT20	5.0	8.0	4.00	2.25
K	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
K	7684	0	7664	0	0	5-8	5-8
F	5556	0	5556	0	0	HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	6183	3610 / 0	1290 / 0	0 / 0	0 / 0	1283 / 0	0 / 0
F	4513	2584 / 0	976 / 0	0 / 0	0 / 0	953 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.96 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO			
A- B	-8216 / 0	-84.3	-84.3	0.13 (1)	4.96	J- B	0 / 3332	0.18 (1)	
B- C	-6006 / 0	-84.3	-84.3	0.05 (1)	5.65	B- H	-3110 / 0	0.43 (1)	
C- D	-6007 / 0	-84.3	-84.3	0.05 (1)	5.66	H- C	0 / 6366	0.34 (1)	
D- E	-7372 / 0	-84.3	-84.3	0.11 (1)	5.19	H- D	-1925 / 0	0.27 (1)	
K- A	-5839 / 0	0.0	0.0	0.13 (1)	7.22	G- D	0 / 1916	0.10 (1)	
F- E	-5475 / 0	0.0	0.0	0.12 (1)	7.39	A- J	0 / 6914	0.37 (1)	
						G- E	0 / 6241	0.33 (1)	
K- L	0 / 0	-28.0	-28.0	0.31 (1)	10.00				
L- M	0 / 0	-28.0	-28.0	0.31 (1)	10.00				
M- N	0 / 0	-28.0	-28.0	0.31 (1)	10.00				
N- J	0 / 0	-28.0	-28.0	0.31 (1)	10.00				
J- O	0 / 6836	-28.0	-28.0	0.37 (1)	10.00				
O- I	0 / 6836	-28.0	-28.0	0.37 (1)	10.00				
I- P	0 / 6836	-28.0	-28.0	0.37 (1)	10.00				
P- H	0 / 6836	-28.0	-28.0	0.37 (1)	10.00				
H- Q	0 / 6135	-28.0	-28.0	0.21 (1)	10.00				
Q- G	0 / 6135	-28.0	-28.0	0.21 (1)	10.00				
G- F	0 / 0	-28.0	-28.0	0.02 (2)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE
G	13-0-12	-2517	-2517	—	BACK	VERT	TOTAL
L	1-0-12	-1452	-1452	—	BACK	VERT	TOTAL
M	3-0-12	-1452	-1452	—	BACK	VERT	TOTAL
N	5-0-12	-1452	-1452	—	BACK	VERT	TOTAL
O	7-0-12	-1452	-1452	—	BACK	VERT	TOTAL
P	9-0-12	-1452	-1452	—	BACK	VERT	TOTAL
Q	11-0-12	-1336	-1336	—	BACK	VERT	TOTAL

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.13 (A-B:1) , BC=0.37 (H-J:1) , WB=0.43 (B-H:1) , SSI=0.54 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

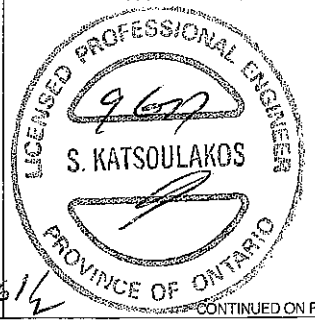
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	622	2264	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (G) (INPUT = 0.90)
JSI METAL= 0.59 (J) (INPUT = 1.00)



DWG NO. TAM 50002-17
STRUCTURAL
COMPONENT ONLY

P6/6

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285812	T170A	1	3	TRUSS DESC.	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1452.1 lbs FACTORED DOWN AT 1-0-12, 1452.1 lbs FACTORED DOWN AT 3-0-12, 1452.1 lbs FACTORED DOWN AT 5-0-12, 1452.1 lbs FACTORED DOWN AT 7-0-12, 1452.1 lbs FACTORED DOWN AT 9-0-12, AND 1337.7 lbs FACTORED DOWN AT 11-0-12, AND 2517.0 lbs FACTORED DOWN AT 13-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
9607
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM50002-17
STRUCTURAL
COMPONENT ONLY

p6 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285817	T171	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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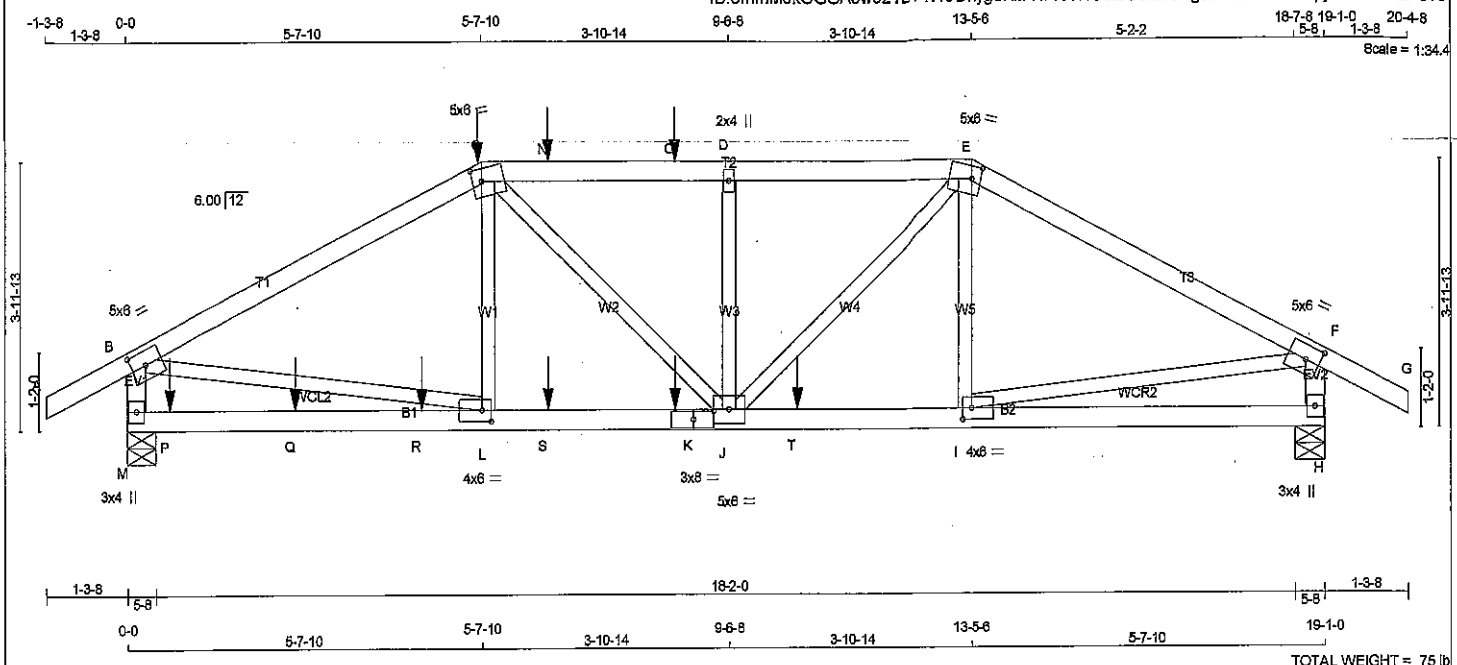

TOTAL WEIGHT = 75 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.50	2.75
C	TTWW-m	MT20	5.0	6.0	2.25	1.75
D	TMW+w	MT20	2.0	4.0		
E	TTWW-m	MT20	5.0	6.0	2.25	1.75
F	TMVW-t	MT20	5.0	6.0	2.50	2.75
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	6.0	2.00	1.75
J	BMWWW-t	MT20	5.0	8.0		
K	BS-t	MT20	3.0	8.0	1.50	3.75
L	BMWW-t	MT20	4.0	6.0	2.00	1.75
M	BMV1+p	MT20	3.0	4.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 201.8 lbs FACTORED DOWN AT 5-7-10, AND 38.3 lbs FACTORED DOWN AT 6-8-4, AND 38.3 lbs FACTORED DOWN AT 8-8-4 ON TOP CHORD, AND 40.7 lbs FACTORED DOWN AT 8-4, 33.0 lbs FACTORED DOWN AT 2-8-4, 33.0 lbs FACTORED DOWN AT 4-8-4, 33.0 lbs FACTORED DOWN AT 6-8-4, AND 33.0 lbs FACTORED DOWN AT 8-8-4, AND 733.1 lbs FACTORED DOWN AT 10-7-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1773	0	1773	0	0	5-8	5-8
H	1708	0	1708	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1436	830 / 0	305 / 0	0 / 0	0 / 0	301 / 0	0 / 0
H	1371	811 / 0	279 / 0	0 / 0	0 / 0	281 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.71 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 26	-84.3	-84.3	0.12 (1)	10.00	L- C	-156 / 218	0.05 (3)
B- C	-2265 / 0	-84.3	-84.3	0.71 (1)	3.71	C- J	0 / 821	0.20 (1)
C- N	-2617 / 0	-84.3	-84.3	0.34 (1)	3.91	J- D	-435 / 0	0.11 (1)
N- O	-2617 / 0	-84.3	-84.3	0.34 (1)	3.91	J- E	0 / 836	0.21 (1)
O- D	-2617 / 0	-84.3	-84.3	0.34 (1)	3.91	I- E	0 / 231	0.06 (3)
D- E	-2617 / 0	-84.3	-84.3	0.34 (1)	3.91	B- L	0 / 2047	0.51 (1)
E- F	-2244 / 0	-84.3	-84.3	0.71 (1)	3.73	I- F	0 / 2028	0.50 (1)
F- G	0 / 26	-84.3	-84.3	0.12 (1)	10.00			
M- B	-1669 / 0	0.0	0.0	0.19 (1)	6.38			
H- F	-1657 / 0	0.0	0.0	0.18 (1)	6.39			
M- P	0 / 0	-28.0	-28.0	0.30 (2)	10.00			
P- Q	0 / 0	-28.0	-28.0	0.30 (2)	10.00			
Q- R	0 / 0	-28.0	-28.0	0.30 (2)	10.00			
R- L	0 / 0	-28.0	-28.0	0.30 (2)	10.00			
L- S	0 / 2020	-28.0	-28.0	0.68 (1)	10.00			
S- K	0 / 2020	-28.0	-28.0	0.68 (1)	10.00			
K- J	0 / 2020	-28.0	-28.0	0.68 (1)	10.00			
J- T	0 / 2007	-28.0	-28.0	0.96 (1)	10.00			
T- I	0 / 2007	-28.0	-28.0	0.96 (1)	10.00			
I- H	0 / 0	-28.0	-28.0	0.31 (2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	5-7-10	-19	-21	—	FRONT	VERT	DEAD
C	5-7-10	-183	-183	—	FRONT	VERT	SNOW
K	6-8-4	-19	-33	—	BACK	VERT	TOTAL
N	6-8-4	-38	-38	—	BACK	VERT	TOTAL
O	8-8-4	-38	-38	—	BACK	VERT	TOTAL
P	8-4	-23	-41	—	BACK	VERT	TOTAL
Q	2-8-4	-19	-33	—	BACK	VERT	TOTAL
R	4-8-4	-19	-33	—	BACK	VERT	TOTAL
S	6-8-4	-19	-33	—	BACK	VERT	TOTAL
T	10-7-8	-733	-733	—	BACK	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.64")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.64")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.71/1.00 (B-C:1) , BC=0.96/1.00 (I-J:1) , WB=0.51/1.00 (B-L:1) , SSI=0.51/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (C) (INPUT = 0.90)
JSI METAL= 0.86 (K) (INPUT = 1.00)

DWG NO. TAM2122-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285812	T172	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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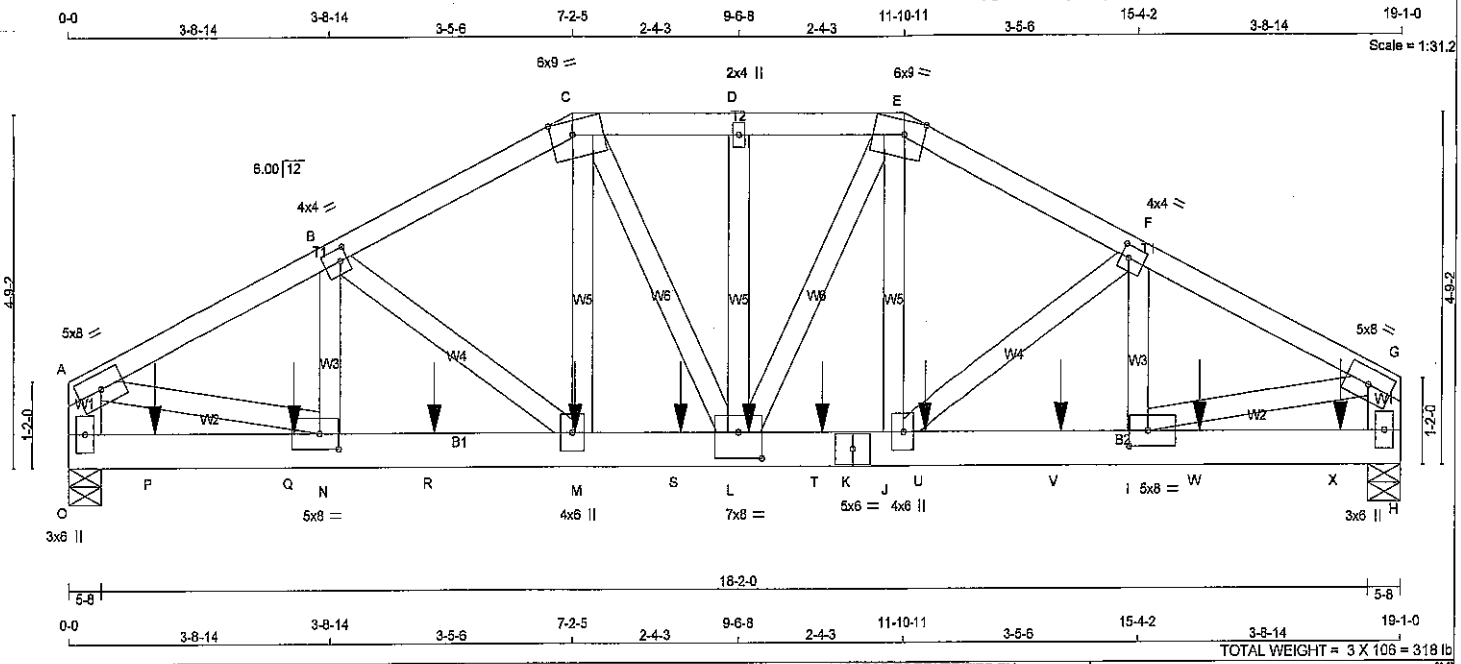

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
O - A	2x6	DRY	No.2	SPF
H - G	2x6	DRY	No.2	SPF
O - K	2x6	DRY	No.2	SPF
K - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY; SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- E	1	12	TOP
E- G	1	12	TOP
O- A	2	12	TOP
H- G	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
O- K	2	7	SIDE(435.9)
K- H	2	7	SIDE(267.8)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	
D- L	2	4	SIDE(1190.3)

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	8.0		
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TTWW-m	MT20	6.0	9.0	2.25	3.75
D	TMW+w	MT20	2.0	4.0		
E	TTWW-m	MT20	6.0	9.0	2.25	3.75
F	TMWW-t	MT20	4.0	4.0	2.00	1.25
G	TMVW-t	MT20	5.0	8.0		
H	BMV1+p	MT20	3.0	6.0		
I	BMWW-t	MT20	5.0	8.0	2.50	3.25
J	BMWW+t	MT20	4.0	6.0		
K	BS-t	MT20	5.0	6.0		
L	BMWWW-t	MT20	7.0	8.0	4.25	4.00
M	BMWW+t	MT20	4.0	6.0		
N	BMWW-t	MT20	5.0	8.0	2.50	3.25
O	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
O	5296	0	5296	0	0	5-8	5-8
H	7751	0	7751	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	4383	2378 / 0	1036 / 0	0 / 0	0 / 0	969 / 0	0 / 0
H	6319	3581 / 0	1391 / 0	0 / 0	0 / 0	1347 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, H.

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.63 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO									
A- B	-7867 / 0	-84.3	-84.3	0.14 (1)	4.18		N- B	-1459 / 0	0.06 (1)
B- C	-8908 / 0	-84.3	-84.3	0.15 (1)	3.96		B- M	0 / 1107	0.06 (1)
C- D	-10009 / 0	-84.3	-84.3	0.16 (1)	3.74		M- C	-129 / 190	0.01 (3)
D- E	-10009 / 0	-84.3	-84.3	0.16 (1)	3.74		C- L	0 / 4355	0.23 (1)
E- F	-10083 / 0	-84.3	-84.3	0.19 (1)	3.72		L- D	-74 / 61	0.01 (1)
F- G	-10468 / 0	-84.3	-84.3	0.22 (1)	3.63		L- E	0 / 1940	0.10 (1)
O- A	-5134 / 0	0.0	0.0	0.11 (1)	7.57		J- E	0 / 2516	0.13 (1)
H- G	-6774 / 0	0.0	0.0	0.14 (1)	6.81		J- F	-445 / 0	0.03 (1)
							I- F	0 / 189	0.01 (2)
O- P	0 / 0	-28.0	-28.0	0.06 (1)	10.00		A- N	0 / 7211	0.39 (1)
P- Q	0 / 0	-28.0	-28.0	0.06 (1)	10.00		I- G	0 / 9564	0.51 (1)
Q- N	0 / 0	-28.0	-28.0	0.06 (1)	10.00				
N- R	0 / 7061	-28.0	-28.0	0.34 (1)	10.00				
R- M	0 / 7061	-28.0	-28.0	0.34 (1)	10.00				
M- S	0 / 7948	-28.0	-28.0	0.39 (1)	10.00				
S- L	0 / 7948	-28.0	-28.0	0.39 (1)	10.00				
L- T	0 / 9091	-28.0	-28.0	0.47 (1)	10.00				
T- K	0 / 9091	-28.0	-28.0	0.47 (1)	10.00				
K- J	0 / 9091	-28.0	-28.0	0.47 (1)	10.00				
J- U	0 / 9365	-28.0	-28.0	0.49 (1)	10.00				
U- V	0 / 9365	-28.0	-28.0	0.49 (1)	10.00				
V- I	0 / 9365	-28.0	-28.0	0.49 (1)	10.00				
I- W	0 / 0	-28.0	-28.0	0.18 (1)	10.00				
W- X	0 / 0	-28.0	-28.0	0.18 (1)	10.00				
X- H	0 / 0	-28.0	-28.0	0.18 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
L	9-8-4	-5528	-5528	---	FRONT	VERT	TOTAL
M	7-2-12	-51	-89	---	FRONT	VERT	TOTAL
P	1-2-12	-51	-89	---	FRONT	VERT	TOTAL
Q	3-2-12	-51	-89	---	FRONT	VERT	TOTAL
R	5-2-12	-51	-89	---	FRONT	VERT	TOTAL
S	8-8-12	-51	-89	---	FRONT	VERT	TOTAL
T	10-8-12	-1024	-1024	---	FRONT	VERT	TOTAL
U	12-2-12	-1024	-1024	---	FRONT	VERT	TOTAL
V	14-2-12	-1024	-1024	---	FRONT	VERT	TOTAL
W	16-2-12	-1024	-1024	---	FRONT	VERT	TOTAL
X	18-2-12	-1026	-1026	---	FRONT	VERT	TOTAL

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.64")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.64")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.22 (F-G:1) , BC=0.49 (I-J:1) , WB=0.51 (G-I:1) , SSI=0.26 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (I) (INPUT = 0.90)
JSI METAL= 0.66 (G) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
9672
S. KATSOULAKOS
PROVINCE OF ONTARIO

P6/6

DWG NO. TAM50004-17
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285812	T172	1	3	TRUSS DESC.	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 88.8 lbs FACTORED DOWN AT 1-2-12, 88.8 lbs FACTORED DOWN AT 3-2-12, 88.8 lbs FACTORED DOWN AT 5-2-12, 88.8 lbs FACTORED DOWN AT 7-2-12, 88.8 lbs FACTORED DOWN AT 8-8-12, 5527.7 lbs FACTORED DOWN AT 9-8-4, 1024.4 lbs FACTORED DOWN AT 10-8-12, 1024.4 lbs FACTORED DOWN AT 12-2-12, 1024.4 lbs FACTORED DOWN AT 14-2-12, AND 1024.4 lbs FACTORED DOWN AT 16-2-12, AND 1026.4 lbs FACTORED DOWN AT 18-2-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
9677
S. KATSOULAKOS
PROVINCE OF ONTARIO

P62

DWG NO. TAM 50004-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285812	T173A	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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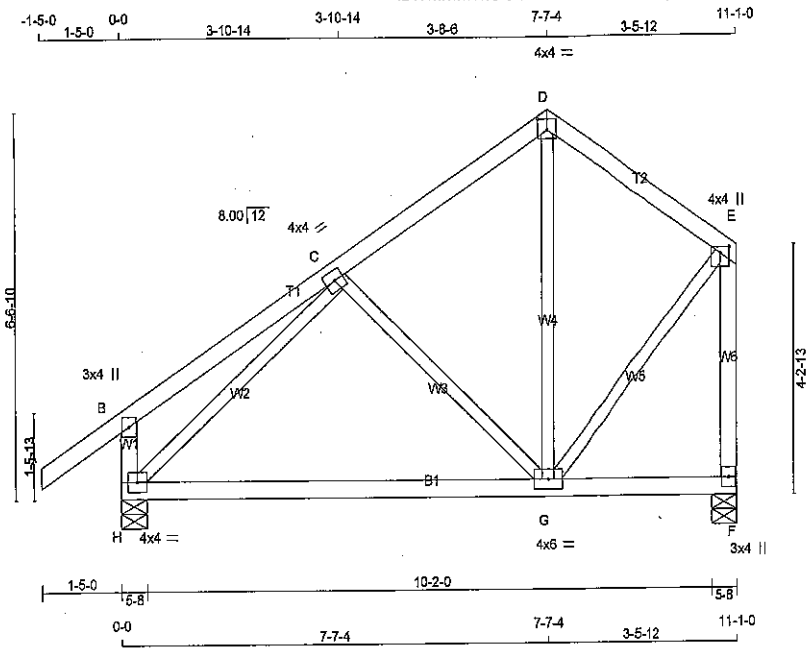

TOTAL WEIGHT = 4 X 53 = 210 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMVW+p	MT20	4.0	4.0	1.25	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	622	0	622	0	0	5-8	5-8
H	748	0	748	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	511	284 / 0	116 / 0	0 / 0	0 / 0	111 / 0	0 / 0
H	596	360 / 0	116 / 0	0 / 0	0 / 0	120 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-84.3	-84.3	0.13 (1)	10.00	C- G	-228 / 11	0.11 (1)
B- C	0 / 21	-84.3	-84.3	0.20 (1)	10.00	G- D	0 / 191	0.04 (3)
C- D	-335 / 0	-84.3	-84.3	0.15 (1)	6.25	H- C	-592 / 0	0.27 (1)
D- E	-314 / 0	-84.3	-84.3	0.13 (1)	6.25	G- E	0 / 394	0.09 (1)
H- B	-248 / 0	0.0	0.0	0.03 (1)	7.81			
F- E	-615 / 0	0.0	0.0	0.18 (1)	7.81			
H- G	0 / 424	-28.0	-28.0	0.40 (2)	10.00			
G- F	0 / 0	-28.0	-28.0	0.36 (3)	10.00			

DWG NO. TAM 50005-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.37")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.37")
CALCULATED VERT. DEFL.(TL) = L/ 645 (0.21")

CSI: TC=0.20 (B-C:1) , BC=0.40 (G-H:2) , WB=0.27 (C-H:1) , SSI=0.17 (G-H:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.49 (H) (INPUT = 0.90)
JSI METAL= 0.20 (C) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285812	T174A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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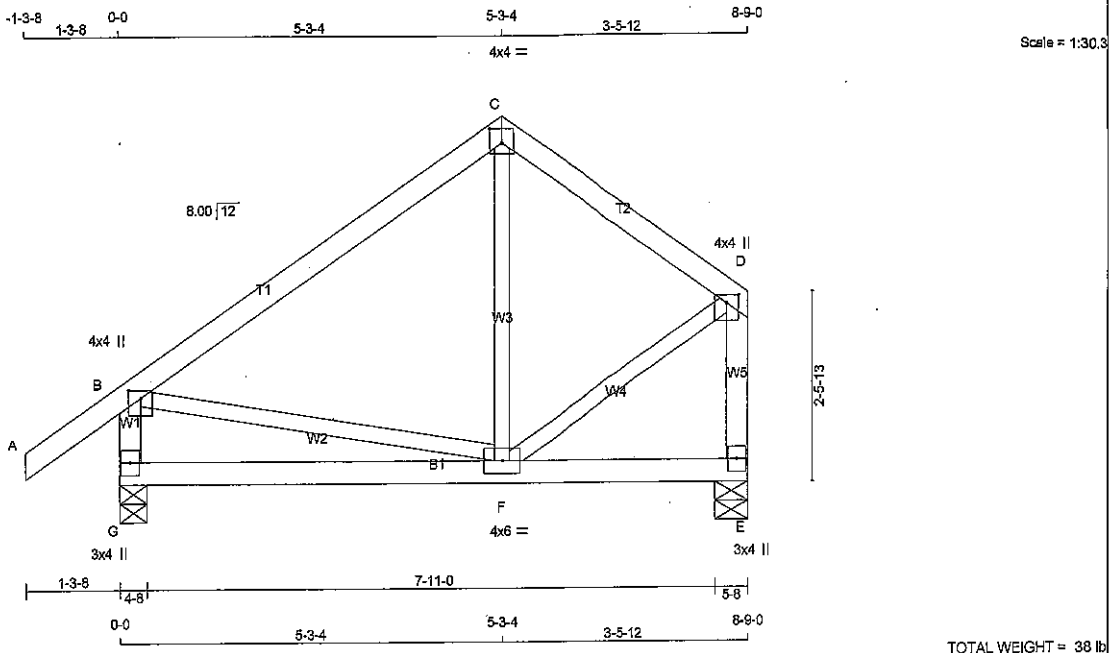

TOTAL WEIGHT = 38 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
G - B	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMVW+p	MT20	4.0	4.0	1.25	2.00
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
G	607	0	607	0	0	4-8	4-8
E	491	0	491	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	482	294 / 0	92 / 0	0 / 0	0 / 0	96 / 0	0 / 0
E	403	224 / 0	92 / 0	0 / 0	0 / 0	87 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 32	-84.3	-84.3	0.11	(1)	10.00	F- C	-43 / 159	0.04 (3)
B- C	-294 / 0	-84.3	-84.3	0.30	(1)	6.25	B- F	0 / 249	0.06 (1)
C- D	-293 / 0	-84.3	-84.3	0.13	(1)	8.25	F- D	0 / 290	0.07 (1)
G- B	-545 / 0	0.0	0.0	0.06	(1)	7.81			
E- D	-465 / 0	0.0	0.0	0.06	(1)	7.81			
G- F	0 / 0	-28.0	-28.0	0.18	(3)	10.00			
F- E	0 / 0	-28.0	-28.0	0.18	(3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.29")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.29")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.30 (B-C:1) , BC=0.18 (E-F:3) , WB=0.07 (D-F:1) , SSI=0.14 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.39 (B) (INPUT = 0.90)
JSI METAL= 0.11 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM5000617
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285812	G174A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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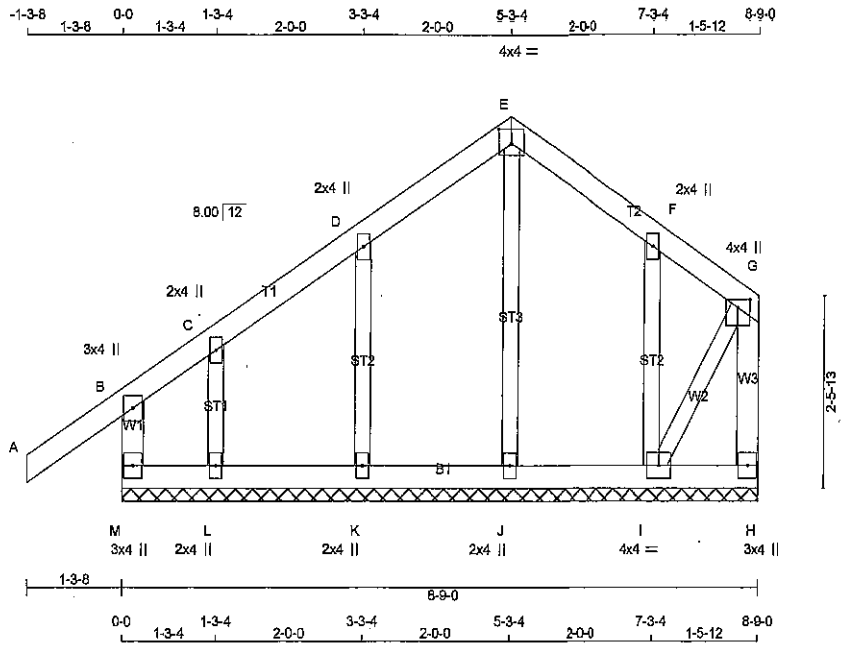

TOTAL WEIGHT = 39 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C, D, F						
C	TMW+w	MT20	2.0	4.0		
E	TTW-p	MT20	4.0	4.0	2.25	2.00
G	TMVW+p	MT20	4.0	4.0	1.25	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMWW1-t	MT20	4.0	4.0		
J, K, L						
J	BMW1+w	MT20	2.0	4.0		
M	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO							FR-TO		
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00		J- E	-150 / 0	0.05 (1)
B- C	-34 / 0	-84.3	-84.3	0.11 (1)	6.25		K- D	-205 / 0	0.04 (1)
C- D	0 / 12	-84.3	-84.3	0.06 (1)	10.00		L- C	-55 / 0	0.01 (1)
D- E	-1 / 0	-84.3	-84.3	0.06 (1)	10.00		I- F	-183 / 0	0.04 (1)
E- F	0 / 0	-84.3	-84.3	0.05 (1)	10.00		I- G	-5 / 0	0.00 (1)
F- G	0 / 17	-84.3	-84.3	0.05 (1)	10.00				
M- B	-227 / 0	0.0	0.0	0.02 (1)	7.81				
H- G	-29 / 0	0.0	0.0	0.00 (1)	7.81				
M- L	0 / 0	-28.0	-28.0	0.02 (2)	10.00				
L- K	-3 / 0	-28.0	-28.0	0.02 (2)	10.00				
K- J	-6 / 0	-28.0	-28.0	0.02 (2)	10.00				
J- I	-6 / 0	-28.0	-28.0	0.02 (2)	10.00				
I- H	0 / 0	-28.0	-28.0	0.02 (2)	10.00				

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM50022-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11 (A-B:1) , BC=0.02 (J-K:2) , WB=0.05 (E-J:1) , SSI=0.08 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (D) (INPUT = 0.90)
JSI METAL= 0.05 (D) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285812	T175	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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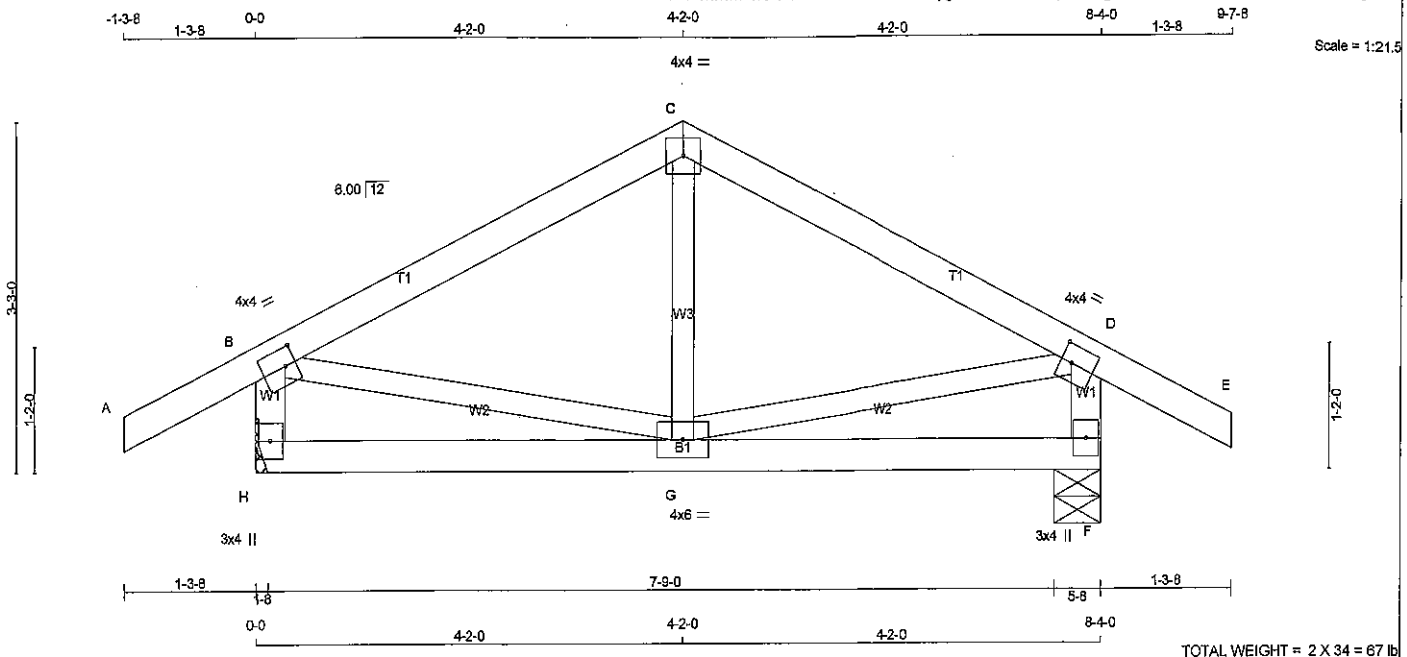

TOTAL WEIGHT = 2 X 34 = 67 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TTW-p	MT20	4.0	4.0		
D	TMVW-t	MT20	4.0	4.0	2.00	1.25
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	
H	582	0	582	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
F	582	0	582	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	462	283 / 0	88 / 0	0 / 0	0 / 0	91 / 0	0 / 0
F	462	283 / 0	88 / 0	0 / 0	0 / 0	91 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS		
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 26	-84.3	-84.3	0.11 (1)	10.00	G- C	-2 / 170	0.04 (3)
B- C	-391 / 0	-84.3	-84.3	0.19 (1)	6.25	B- G	0 / 357	0.08 (1)
C- D	-391 / 0	-84.3	-84.3	0.19 (1)	6.25	G- D	0 / 357	0.08 (1)
D- E	0 / 26	-84.3	-84.3	0.11 (1)	10.00			
H- B	-536 / 0	0.0	0.0	0.05 (1)	7.81			
F- D	-536 / 0	0.0	0.0	0.05 (1)	7.81			
H- G	0 / 0	-28.0	-28.0	0.14 (3)	10.00			
G- F	0 / 0	-28.0	-28.0	0.14 (3)	10.00			

DWG NO. TAM 5000217
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.28")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.28")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.19 (B-C:1) , BC=0.14 (F-G:3) , WB=0.08 (B-G:1) , SSI=0.12 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.63 (D) (INPUT = 0.90)
JSI METAL= 0.19 (D) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285812	T175Z	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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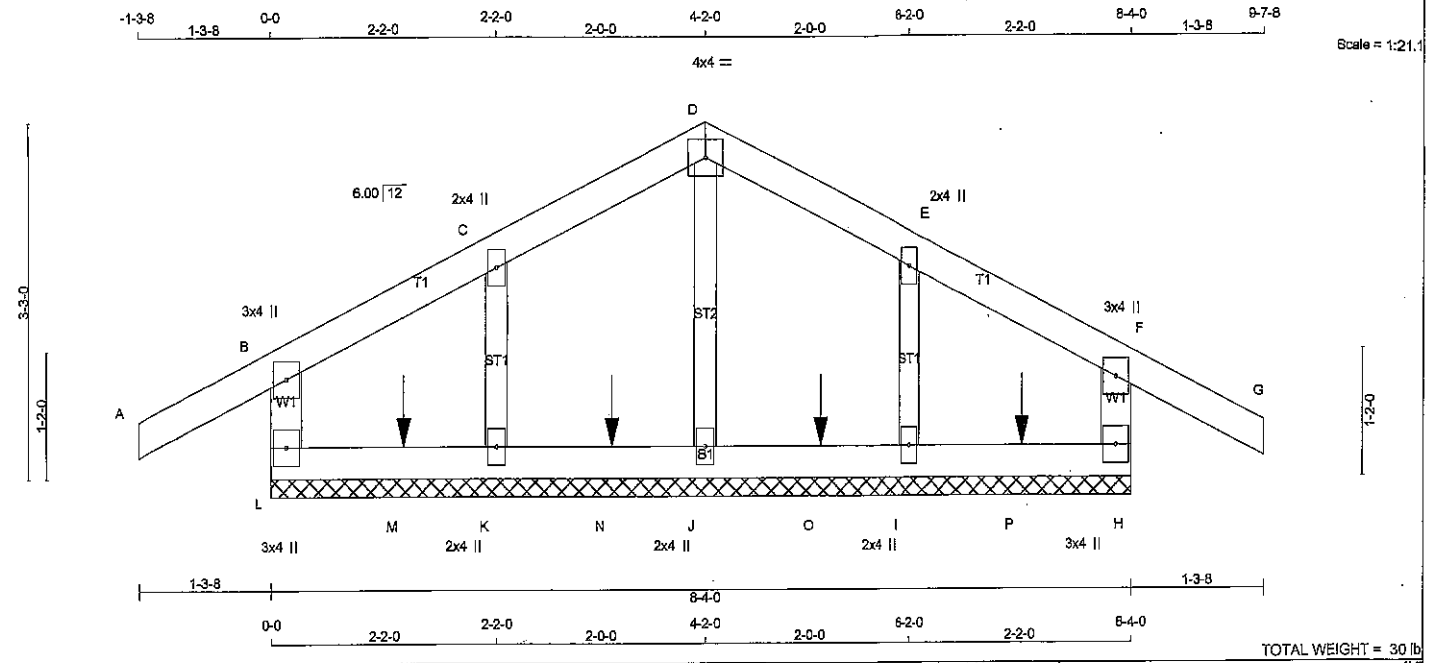

TOTAL WEIGHT = 30 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMW+w	MT20	2.0	4.0		
D	TTW-p	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BMV1+p	MT20	3.0	4.0		
I, J, K						
I	BMW1+w	MT20	2.0	4.0		
L	BMV1+p	MT20	3.0	4.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 11.1 lbs FACTORED DOWN AT 1-3-4, 11.1 lbs FACTORED DOWN AT 3-3-4, AND 11.1 lbs FACTORED DOWN AT 5-3-4, AND 11.1 lbs FACTORED DOWN AT 7-3-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	FORCE (LBS)	MAX. FACTORED CSI (LC)
A- B	0 / 26	-84.3	-84.3	0.12 (1)	10.00	J- D	-159 / 1	0.03 (1)
B- C	-24 / 0	-84.3	-84.3	0.05 (1)	6.25	K- C	-172 / 0	0.03 (1)
C- D	-21 / 0	-84.3	-84.3	0.05 (1)	6.25	I- E	-172 / 0	0.03 (1)
D- E	-21 / 0	-84.3	-84.3	0.05 (1)	6.25			
E- F	-24 / 0	-84.3	-84.3	0.05 (1)	6.25			
F- G	0 / 26	-84.3	-84.3	0.12 (1)	10.00			
L- B	-214 / 0	0.0	0.0	0.02 (2)	7.81			
H- F	-214 / 0	0.0	0.0	0.02 (2)	7.81			
L- M	0 / 21	-28.0	-28.0	0.04 (2)	10.00			
M- K	0 / 21	-28.0	-28.0	0.04 (2)	10.00			
K- N	0 / 19	-28.0	-28.0	0.04 (3)	10.00			
N- J	0 / 19	-28.0	-28.0	0.04 (3)	10.00			
J- O	0 / 19	-28.0	-28.0	0.04 (3)	10.00			
O- I	0 / 19	-28.0	-28.0	0.04 (3)	10.00			
I- P	0 / 21	-28.0	-28.0	0.04 (2)	10.00			
P- H	0 / 21	-28.0	-28.0	0.04 (2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
M	1-3-4	-6	-11	—	BACK	VERT	TOTAL
N	3-3-4	-6	-11	—	BACK	VERT	TOTAL
O	5-3-4	-6	-11	—	BACK	VERT	TOTAL
P	7-3-4	-6	-11	—	BACK	VERT	TOTAL

DWG NO. TAM5000817

STRUCTURAL COMPONENT ONLY

(M)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.12 (A-B:1) , BC=0.04 (H-I:2) , WB=0.03 (D-J:1) , SSI=0.09 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.10 (C) (INPUT = 0.90)
JSI METAL= 0.05 (D) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285812	T176	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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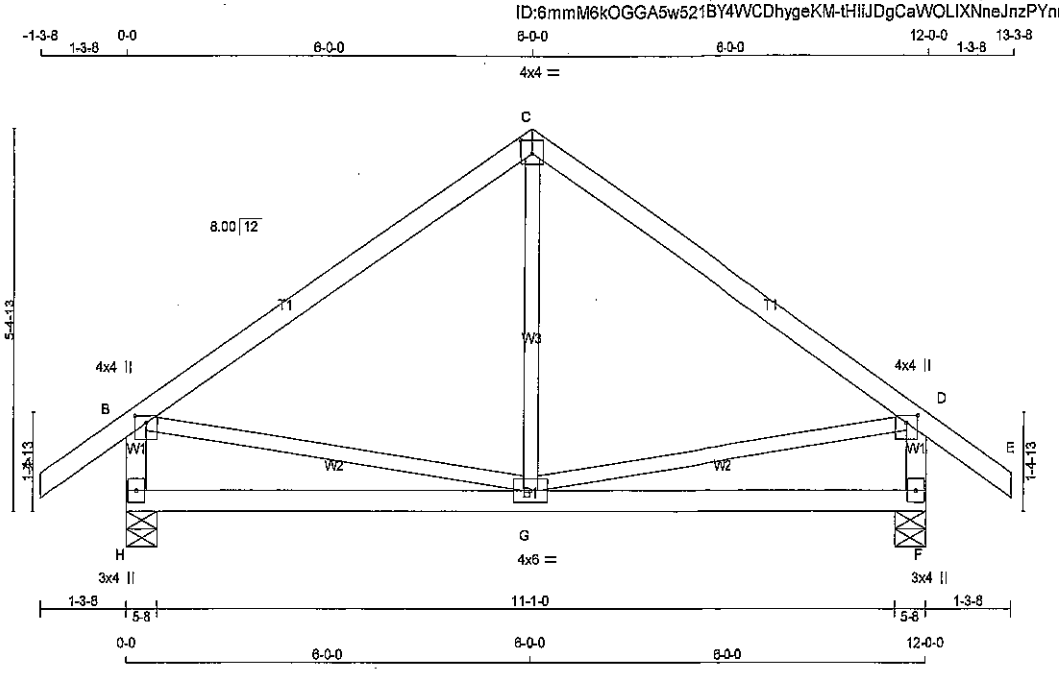

TOTAL WEIGHT = 49 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMVW+p	MT20	4.0	4.0	1.25	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	789	0	789	0	0	5-8	5-8
F	789	0	789	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	631	377 / 0	126 / 0	0 / 0	0 / 0	126 / 0	0 / 0
F	631	377 / 0	126 / 0	0 / 0	0 / 0	126 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO					FR-TO			
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	G- C	0 / 279	0.06 (3)
B- C	-506 / 0	-84.3	-84.3	0.39 (1)	6.25	B- G	0 / 428	0.10 (1)
C- D	-506 / 0	-84.3	-84.3	0.39 (1)	6.25	G- D	0 / 428	0.10 (1)
D- E	0 / 32	-84.3	-84.3	0.11 (1)	10.00			
H- B	-725 / 0	0.0	0.0	0.07 (1)	7.81			
F- D	-725 / 0	0.0	0.0	0.07 (1)	7.81			
H- G	0 / 0	-26.0	-26.0	0.29 (3)	10.00			
G- F	0 / 0	-26.0	-26.0	0.29 (3)	10.00			

[M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.40")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.40")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.39 (B-C:1) , BC=0.29 (G-H:3) , WB=0.10 (B-G:1) , SSI=0.16 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.58 (G) (INPUT = 0.90)
JSI METAL= 0.17 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 50009-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43854	DRWG NO.
285812	G176	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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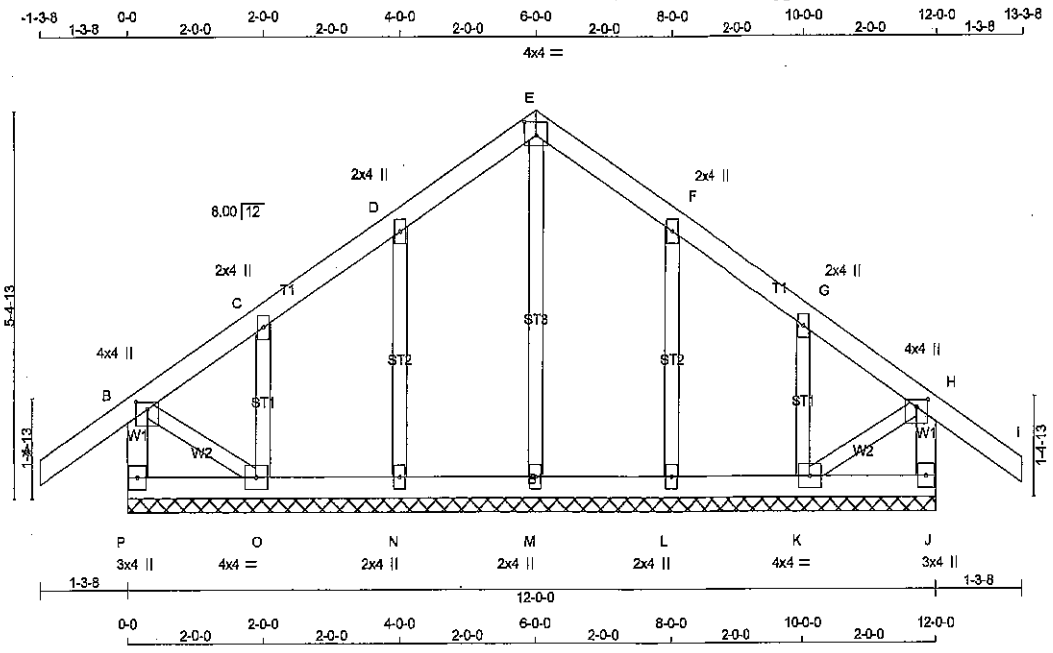

TOTAL WEIGHT = 52 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C, D, F, G						
C	TMW+w	MT20	2.0	4.0		
E	TTW-p	MT20	4.0	4.0	2.25	2.00
H	TMVW+p	MT20	4.0	4.0	1.25	2.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW1-t	MT20	4.0	4.0		
L, M, N						
L	BMW1+w	MT20	2.0	4.0		
O	BMWW1-t	MT20	4.0	4.0		
P	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00
B- C	-38 / 0	-84.3	-84.3	0.11 (1)	6.25
C- D	-6 / 0	-84.3	-84.3	0.05 (1)	10.00
D- E	-19 / 0	-84.3	-84.3	0.05 (1)	6.25
E- F	-19 / 0	-84.3	-84.3	0.05 (1)	6.25
F- G	-6 / 0	-84.3	-84.3	0.05 (1)	10.00
G- H	-38 / 0	-84.3	-84.3	0.11 (1)	6.25
H- I	0 / 32	-84.3	-84.3	0.11 (1)	10.00
P- B	-250 / 0	0.0	0.0	0.03 (1)	7.81
J- H	-250 / 0	0.0	0.0	0.03 (1)	7.81
P- O	0 / 0	-28.0	-28.0	0.03 (2)	10.00
O- N	0 / 12	-28.0	-28.0	0.03 (2)	10.00
N- M	0 / 7	-28.0	-28.0	0.02 (2)	10.00
M- L	0 / 7	-28.0	-28.0	0.02 (2)	10.00
L- K	0 / 12	-28.0	-28.0	0.03 (2)	10.00
K- J	0 / 0	-28.0	-28.0	0.03 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- E	-128 / 0	0.05 (1)
N- D	-201 / 0	0.05 (1)
O- C	-114 / 0	0.02 (1)
L- F	-201 / 0	0.05 (1)
K- G	-114 / 0	0.02 (1)
B- O	0 / 18	0.00 (1)
K- H	0 / 18	0.00 (1)

DWG NO. TAM5002317
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11 (A-B:1) , BC=0.03 (N-O:2) , WB=0.05 (E-M:1) , SSI=0.08 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (H) (INPUT = 0.90)
JSI METAL= 0.05 (D) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285812	T177TCX	5	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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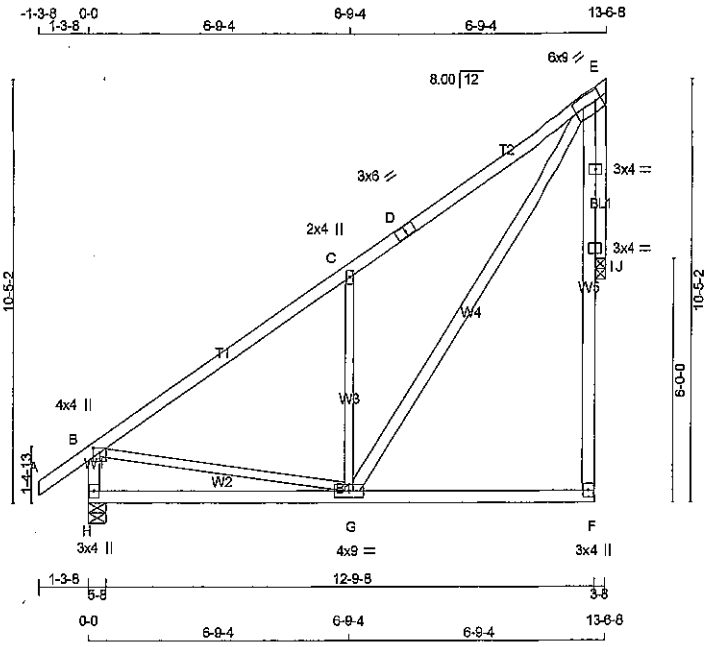

TOTAL WEIGHT = 5 X 75 = 374 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

BEARING NOTE: GAP BETWEEN INSIDE OF TOP CHORD BEARING AND FIRST DIAGONAL OR VERTICAL WEB SHALL NOT EXCEED 0.5 INCHES.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TMW+w	MT20	2.0	4.0		
D	TS-t	MT20	3.0	6.0		
E	TMVWK1-t	MT20	6.0	9.0	3.00	2.00
F	BMV+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BMV1+p	MT20	3.0	4.0		
I	KP-p	MT20	3.0	4.0		
I	KP-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J(E)	736	0	736	0	0	3-8	3-8 (** SEE "BEARING NOTE" **)
H	867	0	867	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J(E)	604	335 / 0	138 / 0	0 / 0	0 / 0	131 / 0	0 / 0
H	696	413 / 0	141 / 0	0 / 0	0 / 0	142 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J(E), H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	
B- C	-637 / 0	-84.3	-84.3	0.50 (1)	6.25	
C- D	-687 / 0	-84.3	-84.3	0.49 (1)	6.25	
D- E	-687 / 0	-84.3	-84.3	0.49 (1)	6.25	
F- I	0 / 116	0.0	0.0	0.10 (1)	10.00	
I- E	0 / 116	0.0	0.0	0.10 (1)	10.00	
H- B	-792 / 0	0.0	0.0	0.08 (1)	7.81	
H- G	0 / 0	-28.0	-28.0	0.36 (3)	10.00	
G- F	0 / 11	-28.0	-28.0	0.36 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- C	-681 / 0	0.37 (1)
G- E	0 / 977	0.16 (1)
B- G	0 / 568	0.13 (1)
E- J	-736 / 0	0.26 (1)
I- J	0 / 26	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.44")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.44")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.50 (B-C:1) , BC=0.36 (F-G:3) , WB=0.37 (C-G:1) , SSI=0.23 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1667	622	2284	1658

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.68 (G) (INPUT = 0.90)
JSI METAL= 0.20 (B) (INPUT = 1.00)

DWG NO. TAM 5001017
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285812	T178TCX	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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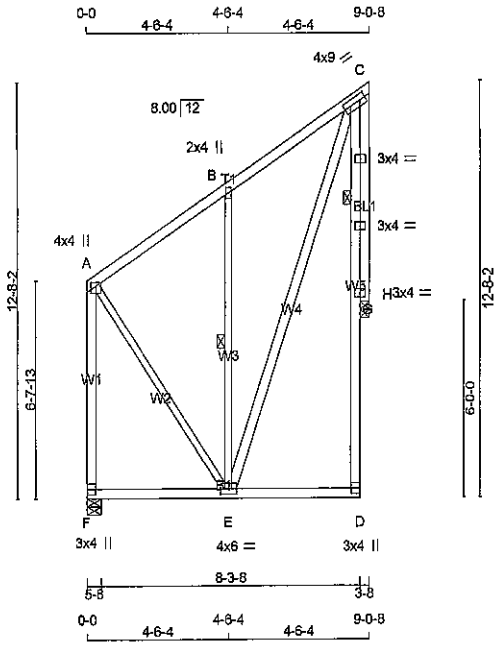

TOTAL WEIGHT = 3 X 78 = 234 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
E - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

BEARING NOTE: GAP BETWEEN INSIDE OF TOP CHORD BEARING AND FIRST DIAGONAL OR VERTICAL WEB SHALL NOT EXCEED 0.5 INCHES.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00
B	TMW+w	MT20	2.0	4.0		
C	TMVWK1-t	MT20	4.0	9.0	2.00	2.25
D	BMV+p	MT20	3.0	4.0		
E	BMWWW-t	MT20	4.0	6.0	2.00	1.50
F	BMV1+p	MT20	3.0	4.0		
G, G, G						
G	KP-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H(C)	463	0	463	0	0	3-8	3-8 (** SEE "BEARING NOTE" **)
F	499	0	499	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H(C)	397	220 / 0	90 / 0	0 / 0	0 / 0	86 / 0	0 / 0
F	410	228 / 0	93 / 0	0 / 0	0 / 0	89 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H(C), F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-G, B-E.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX. CSI	MAX. UNBRAC LENGTH
A- B	-154 / 0	-84.3	-84.3	0.20 (1)	6.25	
B- C	-197 / 0	-84.3	-84.3	0.20 (1)	6.25	
D- G	0 / 77	0.0	0.0	0.05 (1)	10.00	
G- C	0 / 77	0.0	0.0	0.05 (1)	10.00	
F- A	-449 / 0	0.0	0.0	0.44 (1)	7.81	
F- E	0 / 0	-28.0	-28.0	0.16 (3)	10.00	
E- D	0 / 6	-28.0	-28.0	0.16 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-453 / 0	0.28 (1)
E- C	0 / 420	0.07 (1)
A- E	0 / 254	0.06 (1)
C- H	-483 / 0	0.10 (1)
G- H	0 / 11	0.00 (1)

DWG NO. TAM 5001-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.29")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.29")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.44 (A-F:1) , BC=0.16 (D-E:3) , WB=0.28 (B-E:1) , SSI=0.15 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.47 (E) (INPUT = 0.90)
JSI METAL= 0.11 (E) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285815	T400	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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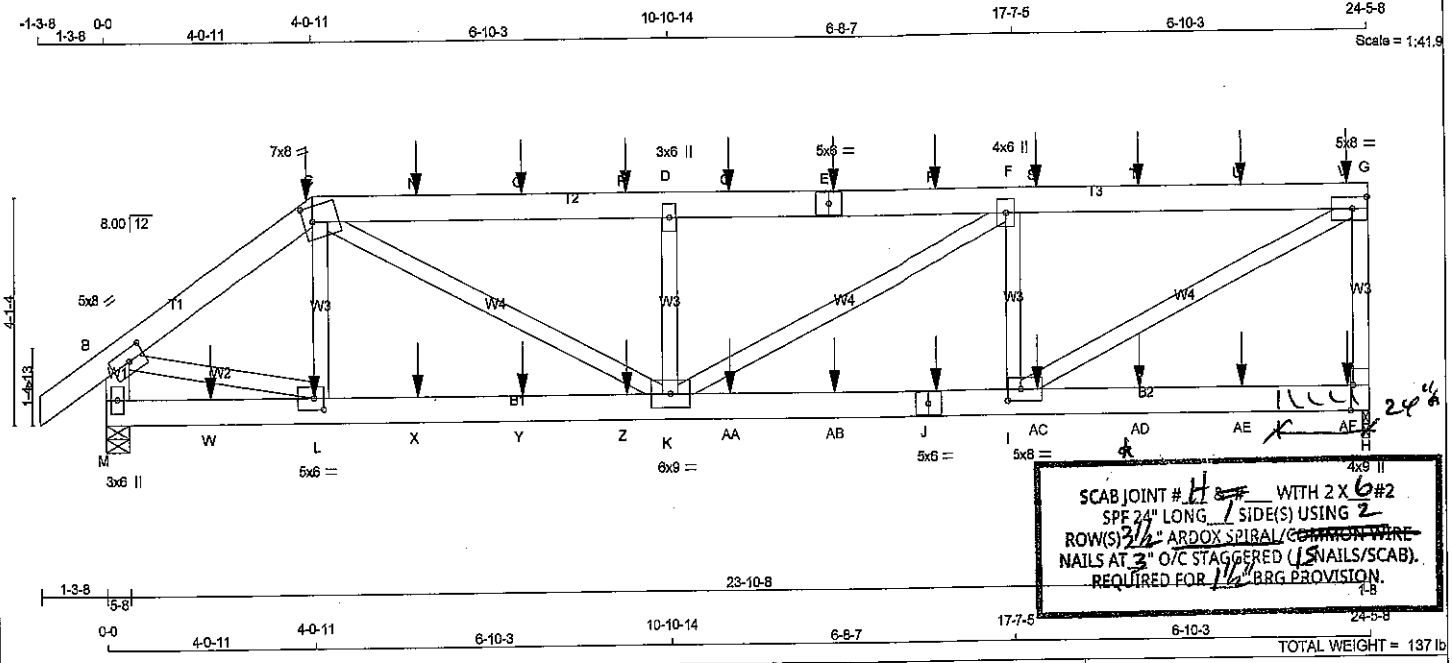

SCAB JOINT # H WITH 2 X 6 #2
SPF 24" LONG 1 SIDE(S) USING 2
ROW(S) 2½" ARDOX SPIRAL/~~COMMON WIRE~~
NAILS AT 3" O/C STAGGERED (15 NAILS/SCAB).
REQUIRED FOR 1½" BRG PROVISION.

TOTAL WEIGHT = 137 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - G	2x6	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x6	DRY	No.2	SPF
M - J	2x6	DRY	2100F 1.6E	SPF
J - H	2x6	DRY	2100F 1.6E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.50	3.75
C	TTWW-m	MT20	7.0	8.0	3.25	2.00
D	TMW+w	MT20	3.0	6.0		
E	TS-t	MT20	5.0	6.0		
F	TMWW+t	MT20	4.0	6.0		
G	TMVW-t	MT20	5.0	8.0	2.50	3.25
H	BMV1+p	MT20	4.0	9.0	Edge	0.50
I	BMWW-t	MT20	5.0	8.0	2.50	3.25
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	6.0	9.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.25
M	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 349.6 lbs FACTORED DOWN AT 4-0-11, 101.4 lbs FACTORED DOWN AT 6-0-12, 101.4 lbs FACTORED DOWN AT 8-0-12, 101.4 lbs FACTORED DOWN AT 10-0-12, 101.4 lbs FACTORED DOWN AT 12-0-12, 101.4 lbs FACTORED DOWN AT 14-0-12, 101.4 lbs FACTORED DOWN AT 16-0-12, 101.4 lbs FACTORED DOWN AT 18-0-12, 101.4 lbs FACTORED DOWN AT 20-0-12, AND 101.4 lbs FACTORED DOWN AT 22-0-12, AND 126.9 lbs FACTORED DOWN AT 24-0-12 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 2-0-12, 69.9 lbs FACTORED DOWN AT 4-0-12, 69.9 lbs FACTORED DOWN AT 6-0-12, 69.9 lbs FACTORED DOWN AT 8-0-12, 69.9 lbs FACTORED DOWN AT 10-0-12, 69.9 lbs FACTORED DOWN AT 12-0-12, 69.9 lbs FACTORED DOWN AT 14-0-12, 69.9 lbs FACTORED DOWN AT 16-0-12, 69.9 lbs FACTORED DOWN AT 18-0-12, 69.9 lbs FACTORED DOWN AT 20-0-12, AND 69.9 lbs FACTORED DOWN AT 22-0-12, AND 84.7 lbs FACTORED DOWN AT 24-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	2344	0	2344	0	0	1-8	4.16
M	2398	0	2398	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1938	1055 / 0	455 / 0	0 / 0	0 / 0	428 / 0	0 / 0
M	1951	1110 / 0	424 / 0	0 / 0	0 / 0	416 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.71 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 33	-84.3	-84.3	0.07 (1)	10.00
B-C	-2808 / 0	-84.3	-84.3	0.17 (1)	4.62
C-N	-3990 / 0	-84.3	-84.3	0.55 (1)	3.74
N-O	-3990 / 0	-84.3	-84.3	0.55 (1)	3.74
O-P	-3990 / 0	-84.3	-84.3	0.55 (1)	3.74
P-D	-3990 / 0	-84.3	-84.3	0.55 (1)	3.74
D-Q	-3991 / 0	-84.3	-84.3	0.57 (1)	3.71
Q-E	-3991 / 0	-84.3	-84.3	0.57 (1)	3.71
E-R	-3991 / 0	-84.3	-84.3	0.57 (1)	3.71
R-F	-3991 / 0	-84.3	-84.3	0.57 (1)	3.71
F-S	-3281 / 0	-84.3	-84.3	0.52 (1)	4.10
S-T	-3281 / 0	-84.3	-84.3	0.52 (1)	4.10
T-U	-3281 / 0	-84.3	-84.3	0.52 (1)	4.10
U-V	-3281 / 0	-84.3	-84.3	0.52 (1)	4.10
V-G	-3281 / 0	-84.3	-84.3	0.52 (1)	4.10
H-G	-2182 / 0	0.0	0.0	0.50 (1)	5.70
M-B	-2354 / 0	0.0	0.0	0.17 (1)	6.62
M-W	0 / 0	-28.0	-28.0	0.10 (2)	10.00
W-L	0 / 0	-28.0	-28.0	0.10 (2)	10.00
L-X	0 / 2322	-28.0	-28.0	0.19 (2)	10.00
X-Y	0 / 2322	-28.0	-28.0	0.19 (2)	10.00
Y-Z	0 / 2322	-28.0	-28.0	0.19 (2)	10.00
Z-K	0 / 2322	-28.0	-28.0	0.19 (2)	10.00
K-AA	0 / 3281	-28.0	-28.0	0.27 (2)	10.00
AA-AB	0 / 3281	-28.0	-28.0	0.27 (2)	10.00
AB-J	0 / 3281	-28.0	-28.0	0.27 (2)	10.00
J-I	0 / 3281	-28.0	-28.0	0.27 (2)	10.00
I-AC	0 / 0	-28.0	-28.0	0.14 (3)	10.00
AC-AD	0 / 0	-28.0	-28.0	0.14 (3)	10.00
AD-AE	0 / 0	-28.0	-28.0	0.14 (3)	10.00
AE-AF	0 / 0	-28.0	-28.0	0.14 (3)	10.00
AF-H	0 / 0	-28.0	-28.0	0.14 (3)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L-C	-186 / 265	0.05 (3)
C-K	0 / 1687	0.33 (1)
K-D	-973 / 0	0.17 (1)
K-F	0 / 808	0.14 (1)
I-F	-1387 / 0	0.24 (1)
I-G	0 / 3717	0.66 (1)
B-L	0 / 2382	0.42 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	4-0-11	-23	-25	---	FRONT	VERT	DEAD
C	4-0-11	-105	-105	---	FRONT	VERT	TOTAL
C	4-0-11	-222	-222	---	FRONT	VERT	SNOW

[M]

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.82")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.82")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.23")

CSI: TC=0.57/1.00 (D-F:1) , BC=0.27/1.00 (I-K:2) , WB=0.86/1.00 (G-I:1) , SSI=0.38/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (I) (INPUT = 0.90)
JSI METAL= 0.61 (G) (INPUT = 1.00)

CONTINUED ON PAGE 2

LICENSED PROFESSIONAL ENGINEER
1216
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 6254 -18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285815	T400	1	1	TRUSS DESC.	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
E	14-0-12	-101	-101	---	FRONT	VERT	TOTAL
J	16-0-12	-40	-70	---	FRONT	VERT	TOTAL
L	4-0-12	-40	-70	---	FRONT	VERT	TOTAL
N	6-0-12	-101	-101	---	FRONT	VERT	TOTAL
O	8-0-12	-101	-101	---	FRONT	VERT	TOTAL
P	10-0-12	-101	-101	---	FRONT	VERT	TOTAL
Q	12-0-12	-101	-101	---	FRONT	VERT	TOTAL
R	16-0-12	-101	-101	---	FRONT	VERT	TOTAL
S	18-0-12	-101	-101	---	FRONT	VERT	TOTAL
T	20-0-12	-101	-101	---	FRONT	VERT	TOTAL
U	22-0-12	-101	-101	---	FRONT	VERT	TOTAL
V	24-0-12	-127	-127	---	FRONT	VERT	TOTAL
W	2-0-12	-40	-70	---	FRONT	VERT	TOTAL
X	6-0-12	-40	-70	---	FRONT	VERT	TOTAL
Y	8-0-12	-40	-70	---	FRONT	VERT	TOTAL
Z	10-0-12	-40	-70	---	FRONT	VERT	TOTAL
AA	12-0-12	-40	-70	---	FRONT	VERT	TOTAL
AB	14-0-12	-40	-70	---	FRONT	VERT	TOTAL
AC	16-0-12	-40	-70	---	FRONT	VERT	TOTAL
AD	20-0-12	-40	-70	---	FRONT	VERT	TOTAL
AE	22-0-12	-40	-70	---	FRONT	VERT	TOTAL
AF	24-0-12	-48	-85	---	FRONT	VERT	TOTAL

DWG NO. TAM 6294-18 PG 4

STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285815	T400A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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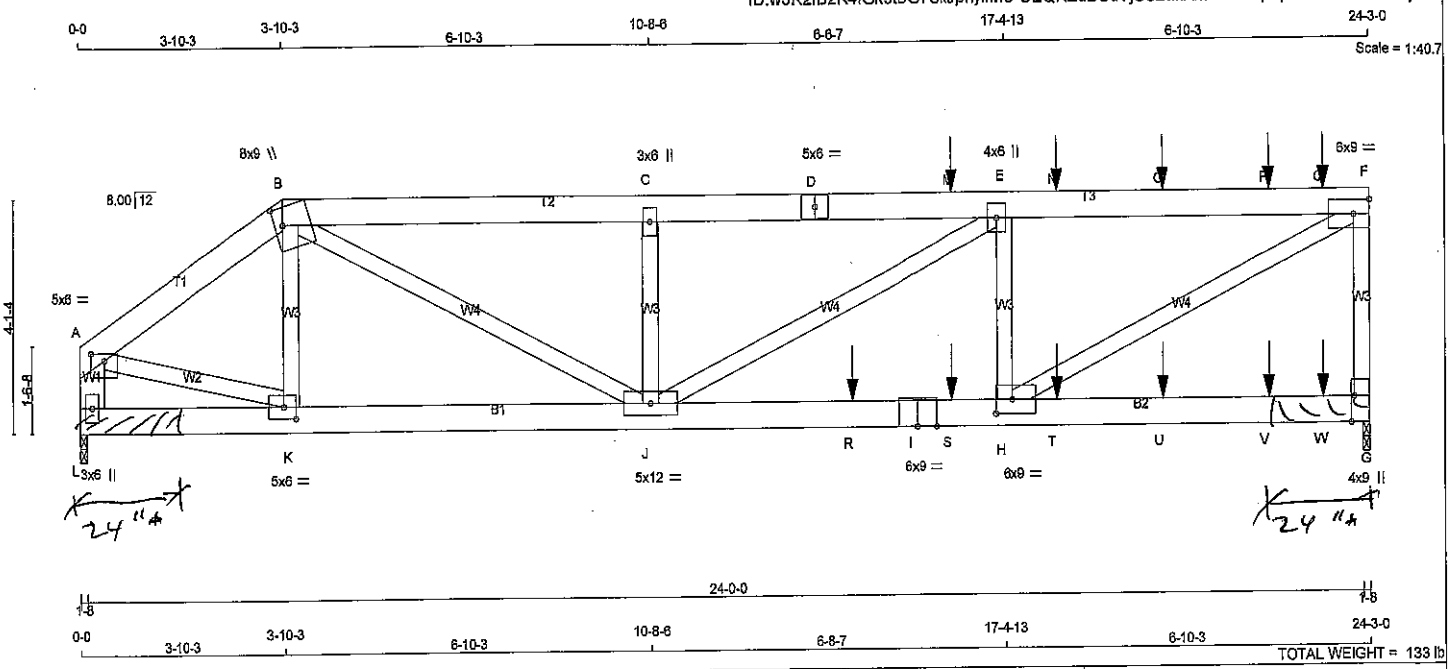

TOTAL WEIGHT = 133 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x6	DRY	No.2	SPF
B - D	2x6	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - A	2x6	DRY	No.2	SPF
L - I	2x6	DRY	2100F 1.8E	SPF
I - G	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	6.0	1.50	3.00
B	TTWW+m	MT20	8.0	9.0	3.75	1.75
C	TMW+w	MT20	3.0	6.0		
D	TS-t	MT20	5.0	6.0		
E	TMWW+t	MT20	4.0	6.0		
F	TMVW-t	MT20	6.0	9.0		Edge
G	BMV1+p	MT20	4.0	9.0	Edge	0.50
H	BMWW-t	MT20	6.0	9.0	3.00	3.75
I	BS-t	MT20	6.0	9.0		
J	BMWWW-t	MT20	5.0	12.0		
K	BMWW-t	MT20	5.0	6.0	2.50	2.75
L	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 101.4 lbs FACTORED DOWN AT 16-7-4, 101.4 lbs FACTORED DOWN AT 18-7-4, 101.4 lbs FACTORED DOWN AT 20-7-4, AND 101.4 lbs FACTORED DOWN AT 22-7-4, AND 107.6 lbs FACTORED DOWN AT 23-7-4 ON TOP CHORD, AND 1316.2 lbs FACTORED DOWN AT 14-8-0, 69.9 lbs FACTORED DOWN AT 16-7-4, 69.9 lbs FACTORED DOWN AT 18-7-4, 69.9 lbs FACTORED DOWN AT 20-7-4, AND 69.9 lbs FACTORED DOWN AT 22-7-4, AND 73.5 lbs FACTORED DOWN AT 23-7-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

SCAB JOINT # G & # L WITH 2 X 6 #2 SPF 24" LONG 1 SIDE(S) USING 2 ROW(S) 3½" ARDOX SPIRAL/~~COMMON WIRE~~ NAILS AT 3" O/C STAGGERED (5 NAILS/SCAB). REQUIRED FOR 1½ SRG PROVISION.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	2744	0	2744	0	0	1-8	1-8
L	2012	0	2012	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	2256	1248 / 0	517 / 0	0 / 0	0 / 0	491 / 0	0 / 0
L	1648	921 / 0	372 / 0	0 / 0	0 / 0	358 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.56 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-2298 / 0	-84.3	-84.3	0.15 (1)		5.25
B- C	-4386 / 0	-84.3	-84.3	0.44 (1)		3.75
C- D	-4386 / 0	-84.3	-84.3	0.55 (1)		3.59
D- M	-4386 / 0	-84.3	-84.3	0.55 (1)		3.59
M- E	-4386 / 0	-84.3	-84.3	0.55 (1)		3.59
E- N	-4188 / 0	-84.3	-84.3	0.62 (1)		3.56
N- O	-4188 / 0	-84.3	-84.3	0.62 (1)		3.56
O- P	-4188 / 0	-84.3	-84.3	0.62 (1)		3.56
P- Q	-4188 / 0	-84.3	-84.3	0.62 (1)		3.56
Q- F	-4188 / 0	-84.3	-84.3	0.62 (1)		3.56
G- F	-2571 / 0	0.0	0.0	0.61 (1)		5.20
L- A	-1939 / 0	0.0	0.0	0.14 (1)		7.19
L- K	0 / 0	-28.0	-28.0	0.04 (1)		10.00
K- J	0 / 1888	-28.0	-28.0	0.35 (1)		10.00
J- R	0 / 4188	-28.0	-28.0	0.68 (1)		10.00
R- I	0 / 4188	-28.0	-28.0	0.68 (1)		10.00
I- S	0 / 4188	-28.0	-28.0	0.68 (1)		10.00
S- H	0 / 4188	-28.0	-28.0	0.68 (1)		10.00
H- T	0 / 0	-28.0	-28.0	0.31 (2)		10.00
T- U	0 / 0	-28.0	-28.0	0.31 (2)		10.00
U- V	0 / 0	-28.0	-28.0	0.31 (2)		10.00
V- W	0 / 0	-28.0	-28.0	0.31 (2)		10.00
W- G	0 / 0	-28.0	-28.0	0.31 (2)		10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- B	-465 / 0	0.08 (1)
B- J	0 / 2626	0.50 (1)
J- C	-575 / 0	0.10 (1)
J- E	0 / 225	0.04 (1)
H- E	-1060 / 0	0.18 (1)
H- F	0 / 4745	0.84 (1)
A- K	0 / 1970	0.35 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
M	16-7-4	-101	-101	—	BACK	VERT	TOTAL
N	18-7-4	-101	-101	—	BACK	VERT	TOTAL
O	20-7-4	-101	-101	—	BACK	VERT	TOTAL
P	22-7-4	-101	-101	—	BACK	VERT	TOTAL
Q	23-7-4	-108	-108	—	BACK	VERT	TOTAL
R	14-8-0	-1318	-1318	—	BACK	VERT	TOTAL
S	16-7-4	-40	-70	—	BACK	VERT	TOTAL
T	18-7-4	-40	-70	—	BACK	VERT	TOTAL
U	20-7-4	-40	-70	—	BACK	VERT	TOTAL
V	22-7-4	-40	-70	—	BACK	VERT	TOTAL
W	23-7-4	-42	-73	—	BACK	VERT	TOTAL

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.28")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 657 (0.44")

CSI: TC=0.62/1.00 (E-F;1) , BC=0.88/1.00 (H-J;1) , WB=0.84/1.00 (F-H;1) , SSI=0.59/1.00 (H-J;1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.94 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 6295 -18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285813	T570	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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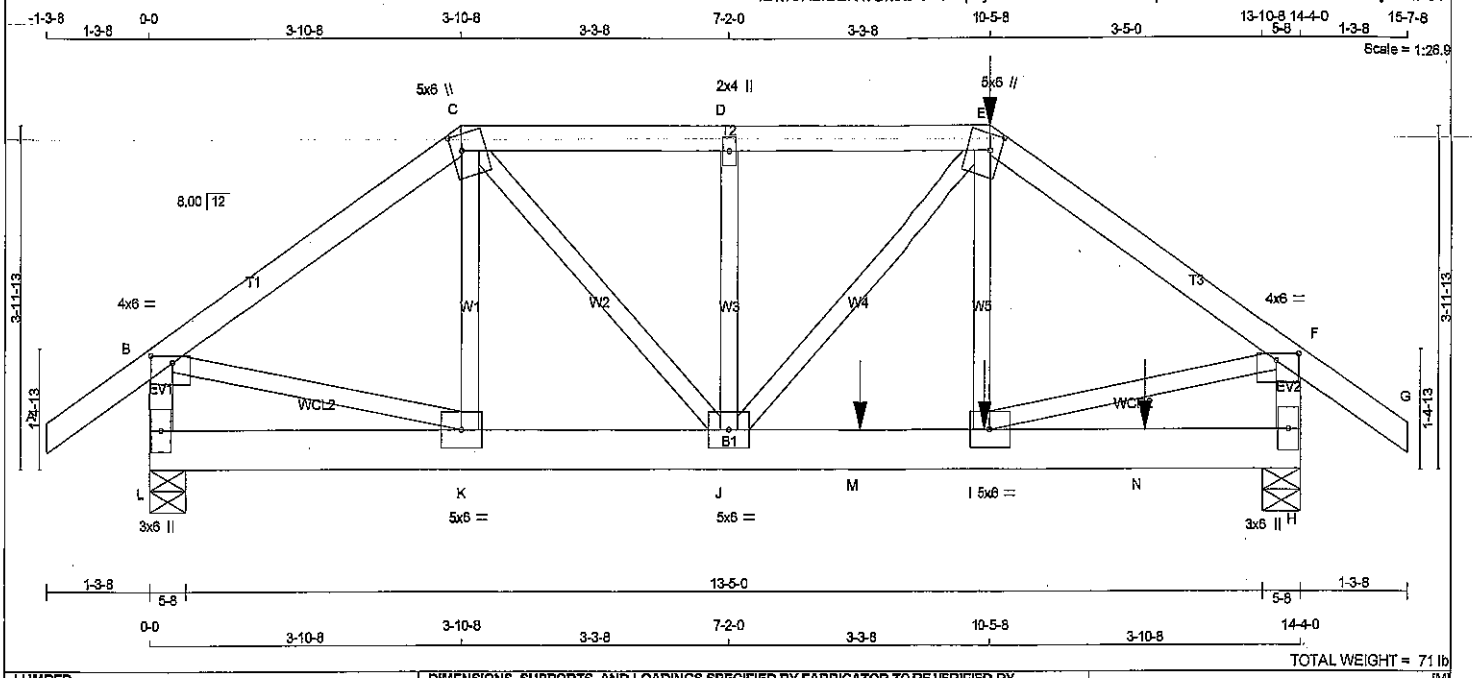

TOTAL WEIGHT = 71 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	6.0	1.00	3.25
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMVW-p	MT20	4.0	6.0	1.00	3.25
H	BMV1+p	MT20	3.0	6.0		
I	BMWW-t	MT20	5.0	6.0		
J	BMWWW-t	MT20	5.0	6.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 225.2 lbs FACTORED DOWN AT 10-5-8 ON TOP CHORD, AND 823.5 lbs FACTORED DOWN AT 8-9-8, AND 33.0 lbs FACTORED DOWN AT 10-4-12, AND 41.0 lbs FACTORED DOWN AT 12-4-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1308	0	1308	0	0	5-8	5-8
H	1623	0	1623	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1036	636 / 0	194 / 0	0 / 0	0 / 0	205 / 0	0 / 0
H	1281	784 / 0	235 / 0	0 / 0	0 / 0	252 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.72 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 32	-84.3	-84.3	0.12 (1)	10.00	K-C	-217 / 59	0.05 (1)
B-C	-1270 / 0	-84.3	-84.3	0.27 (1)	5.36	C-J	0 / 871	0.22 (1)
C-D	-1637 / 0	-84.3	-84.3	0.18 (1)	4.97	J-D	-318 / 0	0.08 (1)
D-E	-1637 / 0	-84.3	-84.3	0.18 (1)	4.97	J-E	0 / 291	0.07 (1)
E-F	-1726 / 0	-84.3	-84.3	0.30 (1)	4.72	I-E	0 / 297	0.07 (2)
F-G	0 / 32	-84.3	-84.3	0.12 (1)	10.00	B-K	0 / 1088	0.27 (1)
L-B	-1250 / 0	0.0	0.0	0.14 (1)	7.16	I-F	0 / 1478	0.37 (1)
H-F	-1599 / 0	0.0	0.0	0.18 (1)	6.49			
L-K	0 / 0	-28.0	-28.0	0.06 (2)	10.00			
K-J	0 / 1049	-28.0	-28.0	0.25 (1)	10.00			
J-M	0 / 1440	-28.0	-28.0	0.57 (1)	10.00			
M-I	0 / 1440	-28.0	-28.0	0.57 (1)	10.00			
I-N	0 / 0	-28.0	-28.0	0.13 (1)	10.00			
N-H	0 / 0	-28.0	-28.0	0.13 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
E	10-5-8	-225	-225	---	FRONT	VERT	TOTAL
I	10-4-12	-19	-33	---	FRONT	VERT	TOTAL
M	8-9-8	-823	-823	---	FRONT	VERT	TOTAL
N	12-4-12	-23	-41	---	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
4/19/18
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.48")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.48")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.30/1.00 (E-F:1) , BC=0.57/1.00 (I-J:1) , WB=0.37/1.00 (F-I:1) , SSI=0.31/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.67 (C) (INPUT = 0.90)
JSI METAL= 0.39 (B) (INPUT = 1.00)

DWG NO. TAM21233-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285813	T580	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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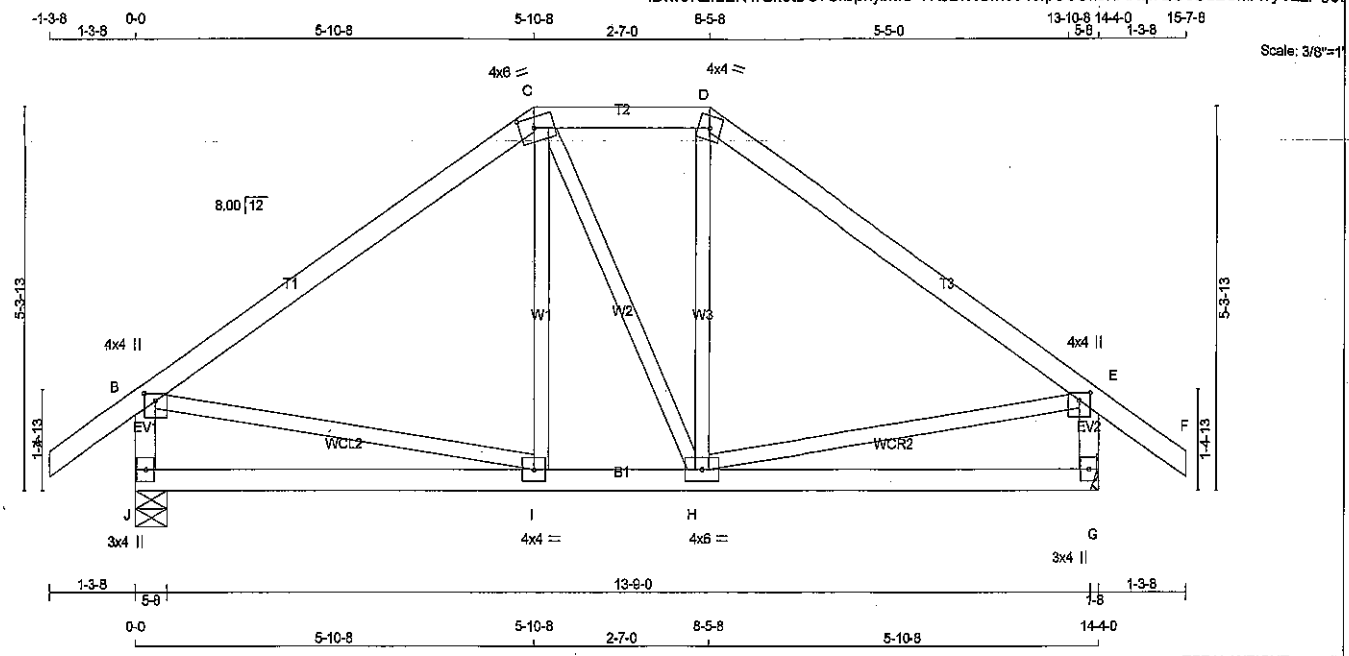

TOTAL WEIGHT = 63 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (tabla is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTWW-m	MT20	4.0	6.0	1.75	2.50
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	4.0	1.25	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	920	0	920	0	0	5-8	5-8
G	920	0	920	0	0	HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	739	437 / 0	151 / 0	0 / 0	0 / 0	152 / 0	0 / 0
G	739	437 / 0	151 / 0	0 / 0	0 / 0	152 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00		I- C	0 / 169	0.04 (3)
B- C	-888 / 0	-84.3	-84.3	0.38 (1)	6.25		C- H	0 / 5	0.00 (3)
C- D	-574 / 0	-84.3	-84.3	0.08 (1)	6.25		H- D	0 / 173	0.04 (3)
D- E	-689 / 0	-84.3	-84.3	0.38 (1)	6.25		B- I	0 / 581	0.13 (1)
E- F	0 / 32	-84.3	-84.3	0.11 (1)	10.00		H- E	0 / 582	0.13 (1)
J- B	-849 / 0	0.0	0.0	0.09 (1)	7.81				
G- E	-849 / 0	0.0	0.0	0.09 (1)	7.81				
J- I	0 / 0	-28.0	-28.0	0.21 (2)	10.00				
I- H	0 / 573	-28.0	-28.0	0.27 (2)	10.00				
H- G	0 / 0	-28.0	-28.0	0.21 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.48")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.48")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.38/1.00 (D-E:1) , BC=0.27/1.00 (H-I:2) , WB=0.13/1.00 (E-H:1) , SSI=0.16/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (H) (INPUT = 0.90)
JSI METAL= 0.21 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4918
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 21234-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285813	T590	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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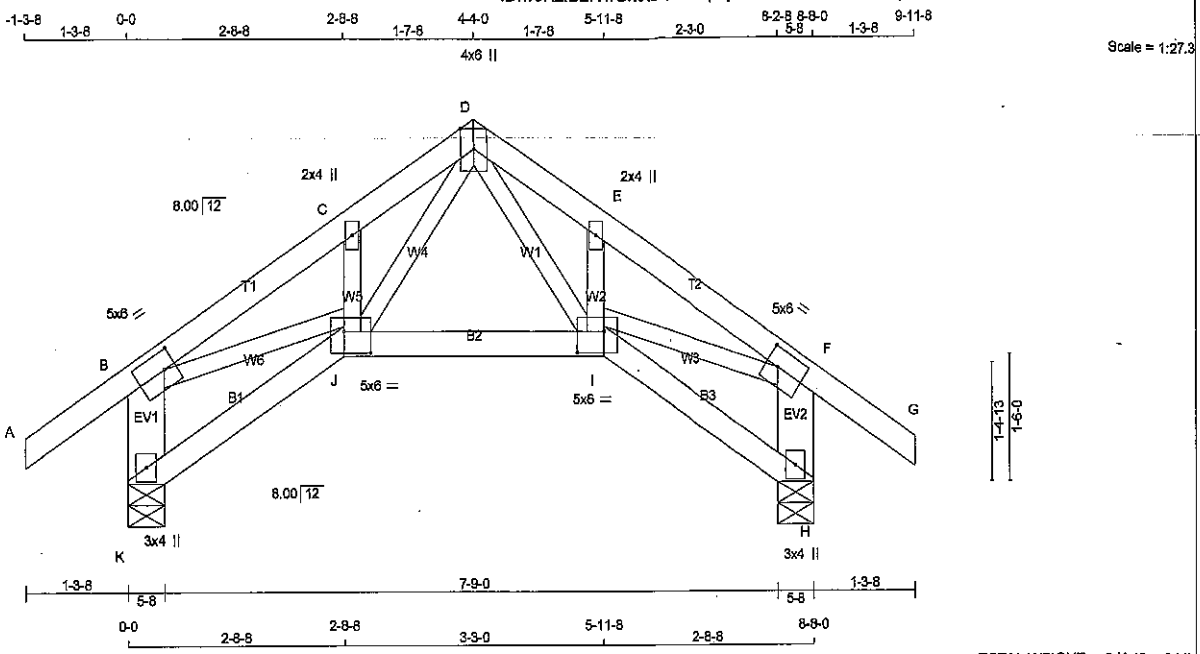

TOTAL WEIGHT = 2 X 42 = 84 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
K - B	2x6	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	1.75
C	TMW+w	MT20	2.0	4.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	5.0	6.0	2.50	1.75
H	BMV1+p	MT20	3.0	4.0		
I	BBWWW-p	MT20	5.0	6.0	3.00	4.00
J	BBWWW-p	MT20	5.0	6.0	3.00	4.00
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	602	0	602	0	0	5-8	5-8
H	602	0	602	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	478	292 / 0	91 / 0	0 / 0	0 / 0	95 / 0	0 / 0
H	478	292 / 0	91 / 0	0 / 0	0 / 0	95 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- B	-564 / 0	0.0	0.0	4	0.04 (1)	7.81	D- I	0 / 460	0.11 (1)
A- B	0 / 32	-84.3	-84.3	1	0.11 (1)	10.00	I- E	-209 / 0	0.03 (1)
B- C	-808 / 0	-84.3	-84.3	1	0.09 (1)	6.25	I- F	0 / 685	0.15 (1)
C- D	-625 / 0	-84.3	-84.3	1	0.03 (1)	6.25	J- D	0 / 460	0.11 (1)
D- E	-825 / 0	-84.3	-84.3	1	0.03 (1)	6.25	J- C	-209 / 0	0.03 (1)
E- F	-808 / 0	-84.3	-84.3	1	0.09 (1)	6.25	B- J	0 / 685	0.15 (1)
F- G	0 / 32	-84.3	-84.3	1	0.11 (1)	10.00			
H- F	-564 / 0	0.0	0.0	4	0.04 (1)	7.81			
K- J	0 / 0	-28.0	-28.0	3	0.06 (3)	10.00			
J- I	0 / 414	-28.0	-28.0	2	0.13 (2)	10.00			
I- H	0 / 0	-28.0	-28.0	3	0.06 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.29")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.29")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.11/1.00 (F-G:1) , BC=0.13/1.00 (I-J:2) , WB=0.15/1.00 (B-J:1) , SSI=0.06/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.59 (I) (INPUT = 0.90)
JSI METAL= 0.23 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4916
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 21235-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285813	T600	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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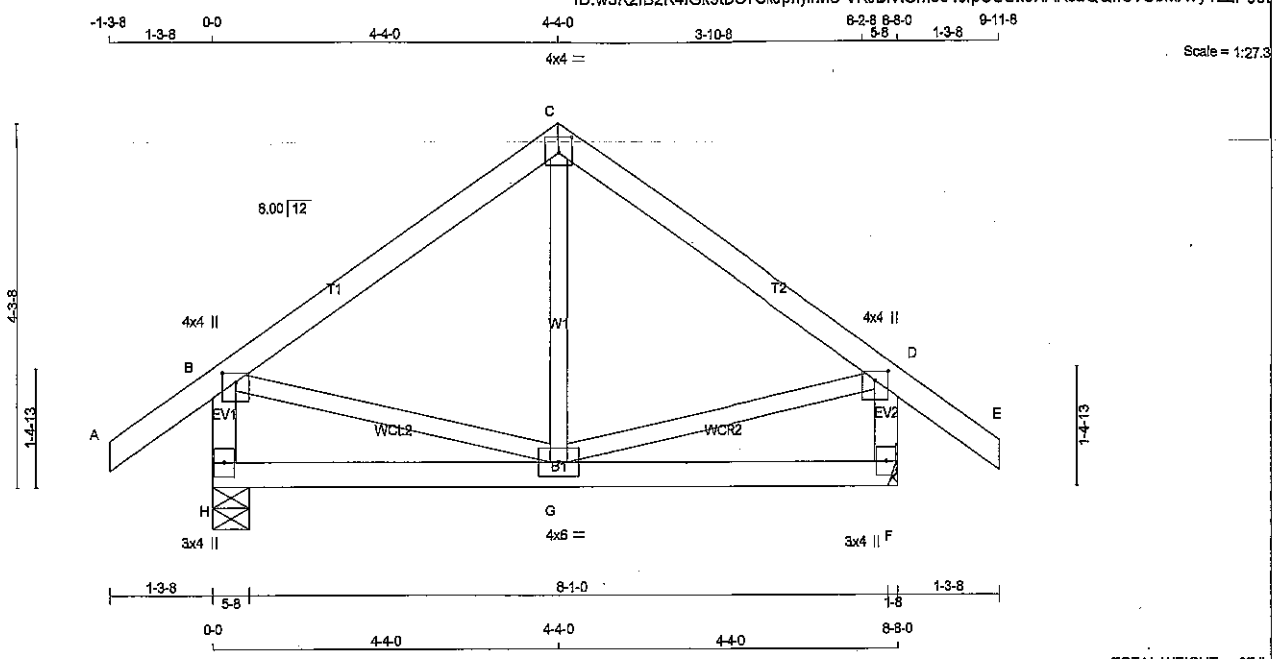

TOTAL WEIGHT = 37 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMVW+p	MT20	4.0	4.0	1.25	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H	602	0	602	0	0	5-8	5-8
F	602	0	602	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	478	292 / 0	91 / 0	0 / 0	0 / 0	95 / 0	0 / 0
F	478	292 / 0	91 / 0	0 / 0	0 / 0	95 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00
B-C	-337 / 0	-84.3	-84.3	0.20 (1)	6.25
C-D	-337 / 0	-84.3	-84.3	0.20 (1)	6.25
D-E	0 / 32	-84.3	-84.3	0.11 (1)	10.00
H-B	-555 / 0	0.0	0.0	0.06 (1)	7.81
F-D	-555 / 0	0.0	0.0	0.06 (1)	7.81
H-G	0 / 0	-28.0	-28.0	0.15 (3)	10.00
G-F	0 / 0	-28.0	-28.0	0.15 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G-C	0 / 183	0.04 (3)
B-G	0 / 289	0.07 (1)
G-D	0 / 289	0.07 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.29")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.29")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.20/1.00 (C-D:1) , BC=0.15/1.00 (F-G:3) , WB=0.07/1.00 (B-G:1) , SSI=0.12/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.41 (G) (INPUT = 0.90)
JSI METAL= 0.12 (B) (INPUT = 1.00)

DWG NO. TAM2123618
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285812	T610	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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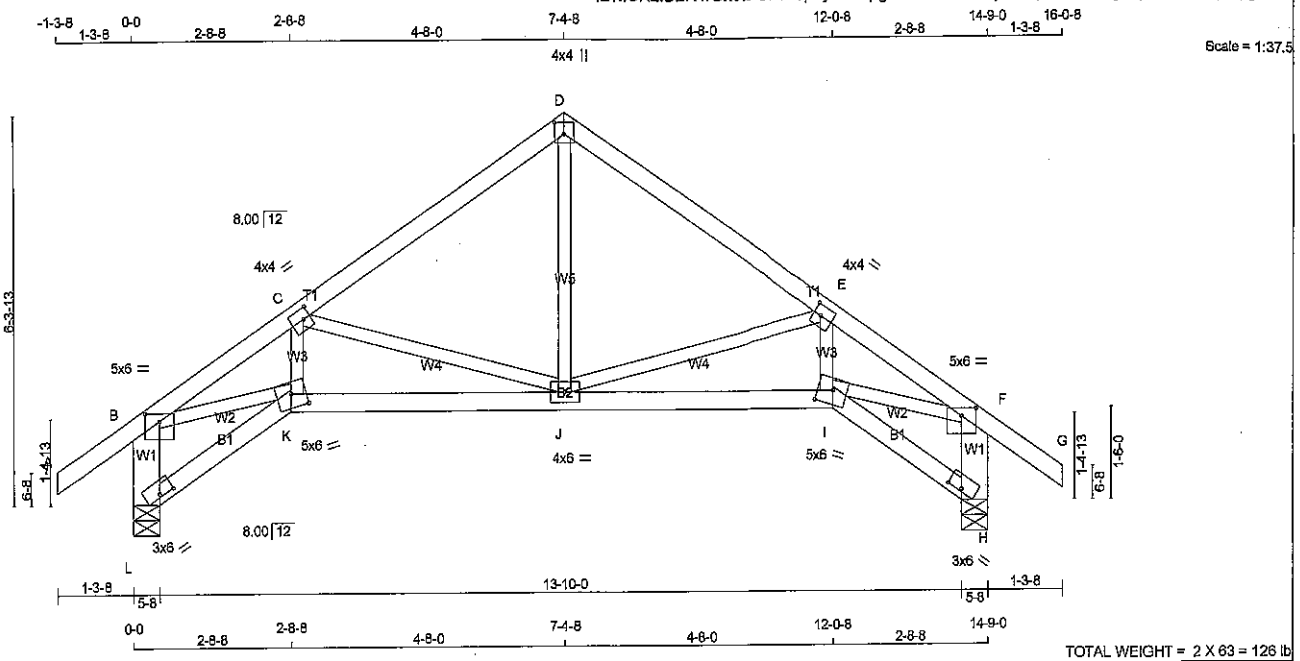

TOTAL WEIGHT = 2 X 63 = 126 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
L - B	2x6	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTW+p	MT20	4.0	4.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0	2.00	1.50
F	TMVW-p	MT20	5.0	6.0	1.50	3.00
H	BVM1-I	MT20	3.0	6.0	0.50	3.00
I	BBWW-m	MT20	5.0	6.0	2.75	3.00
J	BMWWW-t	MT20	4.0	6.0		
K	BBWW-m	MT20	5.0	6.0	2.75	3.00
L	BVM1-I	MT20	3.0	6.0	0.50	3.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	944	0	944	0	0	5-8	5-8
H	944	0	944	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	756	448 / 0	155 / 0	0 / 0	0 / 0	156 / 0	0 / 0
H	758	448 / 0	155 / 0	0 / 0	0 / 0	158 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.99 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L-B	-908 / 0	0.0	0.0	0.06 (1)	7.81	J-D	0 / 598	0.13 (1)
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	J-E	-732 / 0	0.28 (1)
B-C	-1676 / 0	-84.3	-84.3	0.16 (1)	4.99	I-E	0 / 320	0.07 (1)
C-D	-851 / 0	-84.3	-84.3	0.22 (1)	6.25	I-F	0 / 1434	0.32 (1)
D-E	-851 / 0	-84.3	-84.3	0.22 (1)	6.25	C-J	-732 / 0	0.28 (1)
E-F	-1676 / 0	-84.3	-84.3	0.16 (1)	4.99	K-C	0 / 320	0.07 (1)
F-G	0 / 32	-84.3	-84.3	0.11 (1)	10.00	B-K	0 / 1434	0.32 (1)
H-F	-906 / 0	0.0	0.0	0.06 (1)	7.81			
L-K	0 / 0	-28.0	-28.0	0.06 (2)	10.00			
K-J	0 / 1392	-28.0	-28.0	0.33 (2)	10.00			
J-I	0 / 1392	-28.0	-28.0	0.33 (2)	10.00			
I-H	0 / 0	-28.0	-28.0	0.06 (2)	10.00			

DWG NO. TAM50018-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.49")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.49")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.22 (C-D:1) , BC=0.33 (I-J:2) , WB=0.32 (F-I:1) , SSI=0.14 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (B) (INPUT = 0.90)
JSI METAL= 0.35 (I) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285815	T610A	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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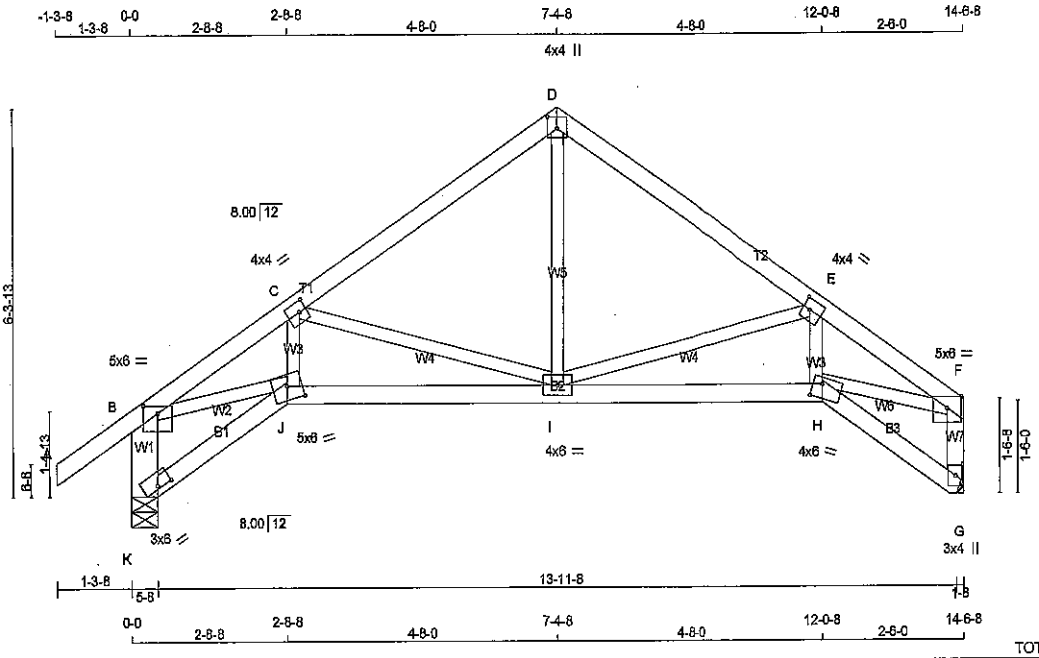

TOTAL WEIGHT = 2 X 60 = 120 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
K - B	2x6	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTW+p	MT20	4.0	4.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0	2.00	1.50
F	TMVW-p	MT20	5.0	6.0	Edge	
G	BMV1+p	MT20	3.0	4.0		
H	BBWW-m	MT20	4.0	6.0	2.75	1.75
I	BMWWW-t	MT20	4.0	6.0		
J	BBWW-m	MT20	5.0	6.0	2.75	3.00
K	BVM1-I	MT20	3.0	8.0	0.50	3.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
K	932	0	932	0	0	5-8	5-8
G	816	0	816	0	0	HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	749	442 / 0	153 / 0	0 / 0	0 / 0	154 / 0	0 / 0
G	670	372 / 0	153 / 0	0 / 0	0 / 0	145 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.02 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- B	-894 / 0	0.0	0.0	0.06 (1)	7.81	I- O	0 / 573	0.13 (1)
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	I- E	-652 / 0	0.25 (1)
B- C	-1649 / 0	-84.3	-84.3	0.16 (1)	5.02	H- E	0 / 261	0.06 (2)
C- D	-826 / 0	-84.3	-84.3	0.22 (1)	6.25	H- F	0 / 1324	0.30 (1)
D- E	-826 / 0	-84.3	-84.3	0.22 (1)	6.25	C- I	-728 / 0	0.28 (1)
E- F	-1552 / 0	-84.3	-84.3	0.16 (1)	5.14	J- C	0 / 316	0.07 (1)
G- F	-761 / 0	0.0	0.0	0.08 (1)	7.81	B- J	0 / 1411	0.32 (1)
K- J	0 / 0	-28.0	-28.0	0.06 (2)	10.00			
J- I	0 / 1370	-28.0	-28.0	0.33 (2)	10.00			
I- H	0 / 1297	-28.0	-28.0	0.32 (2)	10.00			
H- G	0 / 0	-28.0	-28.0	0.05 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.48")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.48")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.22/1.00 (C-D:1) , BC=0.33/1.00 (I-J:2) , WB=0.32/1.00 (B-J:1) , SSI=0.14/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.35 (J) (INPUT = 1.00)

DWG NO. TAM337 / -18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285812	P6	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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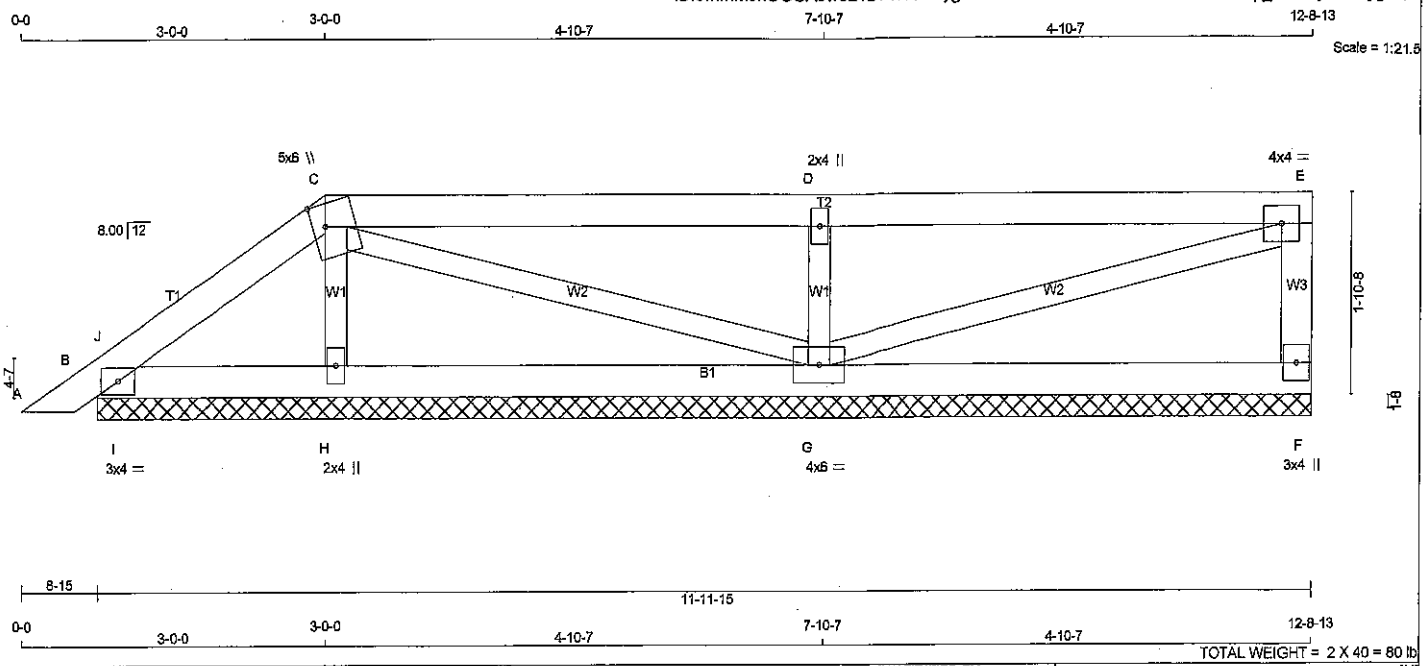

TOTAL WEIGHT = 2 X 40 = 80 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-i	MT20	3.0	4.0		
C	TTWW+m	MT20	5.0	6.0	2.50	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW1-t	MT20	4.0	6.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	210	0	210	0	0	11-11-15	11-11-15
B	194	0	194	0	0	11-11-15	11-11-15
H	321	0	321	0	0	11-11-15	11-11-15
G	677	0	677	0	0	11-11-15	11-11-15

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	173	95 / 0	40 / 0	0 / 0	0 / 0	38 / 0	0 / 0
B	142	106 / 0	14 / 0	0 / 0	0 / 0	22 / 0	0 / 0
H	276	131 / 0	79 / 0	0 / 0	0 / 0	65 / 0	0 / 0
G	550	315 / 0	119 / 0	0 / 0	0 / 0	116 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 14	-84.3	-84.3	0.03 (1)		10.00
B- J	-32 / 0	-84.3	-84.3	0.01 (1)		6.25
J- C	-49 / 0	-84.3	-84.3	0.05 (1)		6.25
C- D	0 / 0	-84.3	-84.3	0.34 (1)		10.00
D- E	0 / 0	-84.3	-84.3	0.34 (1)		10.00
F- E	-156 / 0	0.0	0.0	0.02 (1)		7.81
B- I	0 / 38	-28.0	-28.0	0.05 (1)		10.00
I- H	0 / 38	-28.0	-28.0	0.09 (2)		10.00
H- G	0 / 25	-28.0	-28.0	0.17 (2)		10.00
G- F	0 / 0	-28.0	-28.0	0.17 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-196 / 0	0.03 (1)
C- G	-27 / 0	0.01 (1)
G- D	-512 / 0	0.07 (1)
G- E	0 / 0	0.00 (1)
I- J	-121 / 0	0.00 (1)

DWG NO. TAM5002417
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.34 (C-D:1) , BC=0.17 (G-H:2) , WB=0.07 (D-G:1) , SSI=0.20 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (D) (INPUT = 0.90)
JSI METAL= 0.09 (D) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285812	P60A	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Wed Sep 06 12:11:14 2017 Page 1

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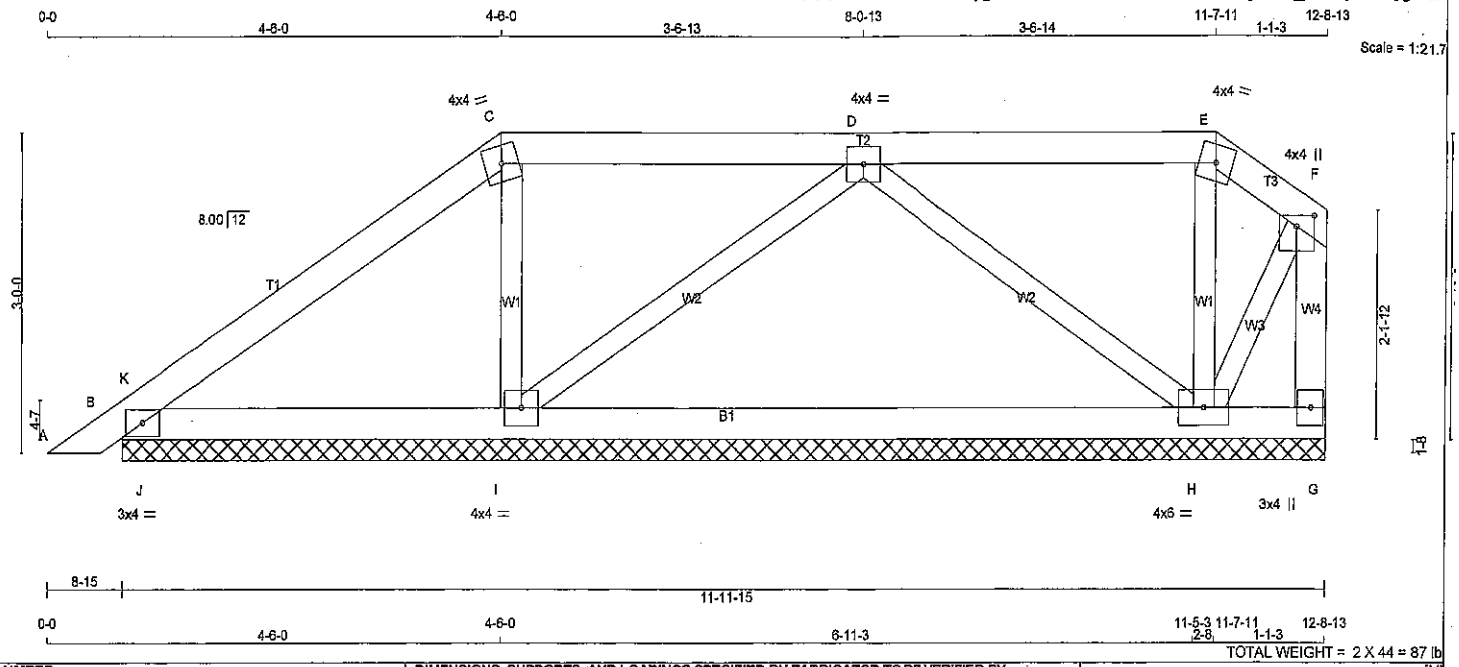


TOTAL WEIGHT = 2 X 44 = 87 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
B - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW-m	MT20	4.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TTW-m	MT20	4.0	4.0		
F	TMVW+p	MT20	4.0	4.0	1.25	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW1-t	MT20	4.0	6.0		
I	BMWW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	-55	0	0	0	-121	11-11-15	11-11-15
B	253	0	253	0	0	11-11-15	11-11-15
I	615	0	615	0	0	11-11-15	11-11-15
H	589	0	589	0	0	11-11-15	11-11-15

PROVIDE ANCHORAGE AT BEARING JOINT G FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	-77	9 / 0	0 / -53	0 / 0	0 / 0	0 / -34	0 / 0
B	189	135 / 0	23 / 0	0 / 0	0 / 0	31 / 0	0 / 0
I	515	269 / 0	129 / 0	0 / 0	0 / 0	118 / 0	0 / 0
H	516	234 / 0	152 / 0	0 / 0	0 / 0	129 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, B, I, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 14	-84.3	-84.3	0.03 (1)	10.00	I- C	-234 / 0	0.04 (1)
B- K	-6 / 66	-84.3	-84.3	0.07 (1)	10.00	H- E	-172 / 0	0.03 (1)
K- C	-31 / 0	-84.3	-84.3	0.14 (1)	6.25	H- F	-31 / 0	0.00 (1)
C- D	-10 / 0	-84.3	-84.3	0.17 (1)	6.25	I- D	-290 / 0	0.09 (1)
D- E	0 / 24	-84.3	-84.3	0.18 (1)	10.00	D- H	-333 / 0	0.11 (1)
E- F	0 / 21	-84.3	-84.3	0.02 (1)	10.00	J- K	-312 / 0	0.00 (1)
G- F	-9 / 0	0.0	0.0	0.00 (1)	7.81			
B- J	0 / 20	-28.0	-28.0	0.13 (1)	10.00			
J- I	0 / 20	-28.0	-28.0	0.23 (2)	10.00			
I- H	0 / 243	-28.0	-28.0	0.26 (2)	10.00			
H- G	0 / 0	-28.0	-28.0	0.25 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.18 (D-E:1) , BC=0.26 (H-I:2) , WB=0.11 (D-H:1) , SSI=0.24 (B-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.35 (H) (INPUT = 0.90)
JSI METAL= 0.08 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
9607
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM50025-17
STRUCTURAL
COMPONENT ONLY

LUL/LUS/LJS/HUS/HHUS/HGUS

Standard and Double-Shear Joist Hangers

This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 258–259.

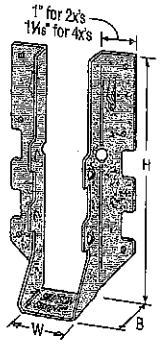
Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 20–24.

Installation:

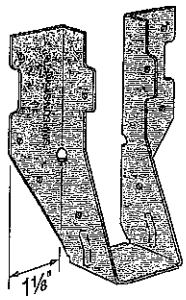
- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

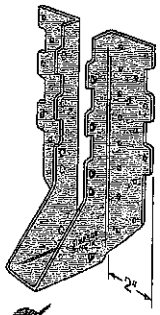
- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on p. 126.



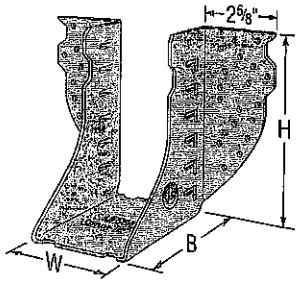
LUS28



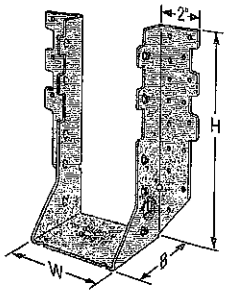
LU26L



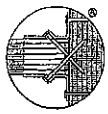
HUS210
(HUS26, HUS28, and HHUS similar)



HGUS28-2

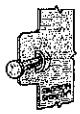


HHUS210-2



Double-Shear Nailing Top View

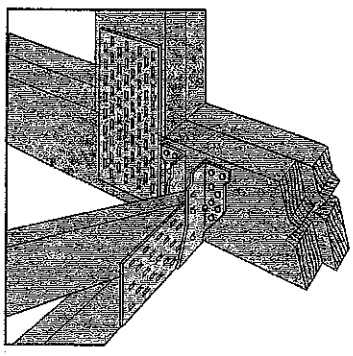
Double-Shear Nailing Side View; Do not bend tab

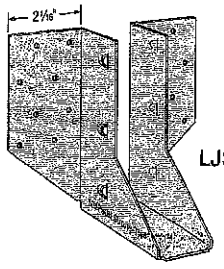


Dome Double-Shear Nailing Side View *(available on some models)*

U.S. Patent 5,603,580

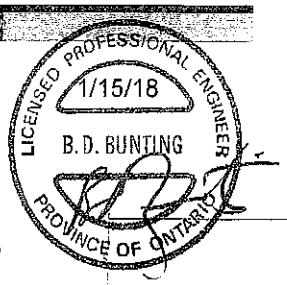
Typical HUS26 Installation with Reduced Heel Height
(Truss Designer to provide fastener quantity for connecting multiple members together)





LJS26DS

LUL/LUS/LJS/HUS/HHUS/HGUS



HHUS/HGUS

See Hanger Options information on pp. 125–127.

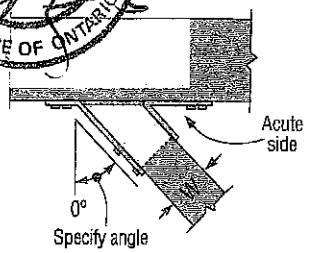
HHUS — Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.62 of the table value
- The joist must be bevel-cut to allow for double-shear nailing

HGUS — Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Joist	Down Resistance	Uplift
W < 2"	Bevel or square cut	0.62 of table value	0.46 of table value
2" < W < 6"	Bevel cut	0.67 of table value	0.41 of table value
2" < W < 6"	Square cut	0.46 of table value	0.41 of table value
W > 6"	Bevel cut	0.75 of table value	0.41 of table value



Top View HHUS Hanger Skewed Right
(joist must be bevel cut)
All joist nails installed on the outside angle *(non-acute side)*.

Standard and Double-Shear Joist Hangers (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.) W	H	B	d_e[3]	Fasteners Header	Joist	Factored Resistance D.Fir-L Uplift (K_D = 1.15) lb. / kN	D.Fir-L Normal (K_D = 1.00) lb. / kN	S-P-F Uplift (K_D = 1.15) lb. / kN	S-P-F Normal (K_D = 1.00) lb. / kN
Single 2x Sizes											
LUS24	18	1⅝	3⅛	1¾	2¼	(4) 10d	(2) 10d	710 / 3.16	1625 / 7.23	645 / 2.87	1155 / 5.14
LU24L	22	1⅝	3	1⅝	2⅛	(4) 10d	(2) 10d x 1½"	360 / 1.60	1020 / 4.54	320 / 1.42	725 / 3.22
LU26L	22	1⅝	5	1⅝	4⅝	(6) 10d	(4) 10d x 1½"	720 / 3.20	1605 / 7.14	645 / 2.87	1140 / 5.07
LUS26	18	1⅝	4¾	1¾	3¾	(4) 10d	(4) 10d	1420 / 6.32	2170 / 9.65	1290 / 5.74	1630 / 7.25
HUS26	16	1⅝	5⅞	3	3⅛	(14) 16d	(6) 16d	2705 / 11.30	4940 / 21.97	2065 / 9.20	3875 / 17.24
LJS26DS	18	1⅝	5	3½	4⅝	(16) 16d	(6) 16d	2055 / 9.14	4265 / 18.97	1460 / 6.49	4115 / 18.31
HGUS26	12	1⅝	5⅞	5	4⅛	(20) 16d	(8) 16d	2685 / 11.96	6625 / 29.51	2685 / 11.96	5700 / 25.35
LU28L	20	1⅝	6¾	1⅝	5⅞	(8) 10d	(6) 10d x 1½"	1140 / 5.07	2185 / 9.72	1020 / 4.54	1550 / 6.89
LUS28	18	1⅝	6⅝	1¾	3¾	(6) 10d	(4) 10d	1420 / 6.32	2520 / 11.21	1290 / 5.74	1790 / 7.96
HUS28	16	1⅝	7⅛	3	6⅛	(22) 16d	(8) 16d	3605 / 16.04	5365 / 23.86	2675 / 11.90	4345 / 19.33
HGUS28	12	1⅝	7⅛	5	6⅛	(36) 16d	(12) 16d	3310 / 14.74	7675 / 34.19	3310 / 14.74	6900 / 30.73
LU210L	20	1⅝	8	1⅝	7⅝	(10) 10d	(6) 10d x 1½"	1140 / 5.07	2495 / 11.10	1020 / 4.54	1770 / 7.87
LUS210	18	1⅝	7¹³⁄₁₆	1¾	3⅞	(8) 10d	(4) 10d	1420 / 6.32	2785 / 12.39	1290 / 5.74	2210 / 9.83

1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.
3. d_e is the distance from the bearing seat to the top joist nail.
4. Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-C-N10TRSSCN and/or see installation notes.
5. **Nails:** 16d = 0.162" dia. x 3½" long. See pp. 27–28 for other nail sizes and information.

Face-Mount Hangers

These products are available with additional corrosion protection. For more information, see p. 24.

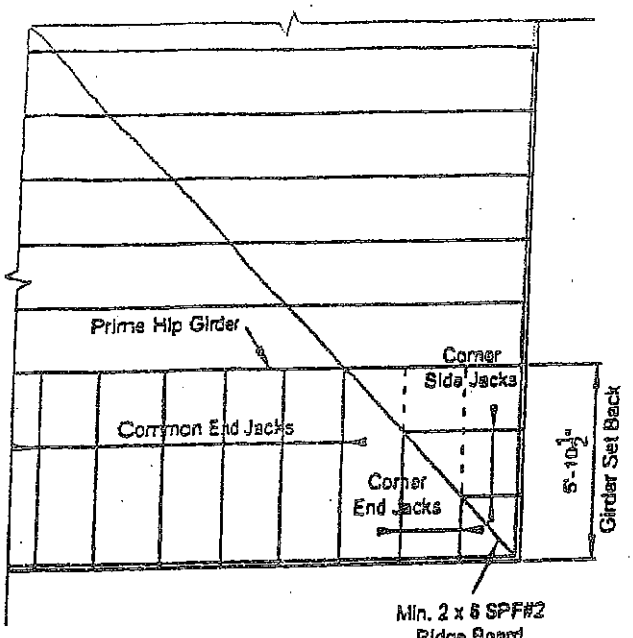
These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

Model No.	Ga.	Dimensions (in.) W	H	B	d_e[3]	Fasteners Header	Joist	Factored Resistance D.Fir-L Uplift (K_D = 1.15) lb. / kN	D.Fir-L Normal (K_D = 1.00) lb. / kN	S-P-F Uplift (K_D = 1.15) lb. / kN	S-P-F Normal (K_D = 1.00) lb. / kN
Double 2x Sizes											
LUS24-2	18	3⅛	3⅛	2	1½	(4) 16d	(2) 16d	835 / 3.71	2020 / 8.99	590 / 2.62	1435 / 6.38
LUS26-2	18	3⅛	4⅞	2	4	(4) 16d	(4) 16d	1720 / 7.65	2595 / 11.54	1545 / 6.87	1920 / 8.54
HHUS26-2	14	3⁵⁄₁₆	5⅜	3	3¹⁵⁄₁₆	(14) 16d	(6) 16d	2850 / 12.68	7335 / 32.63	2065 / 9.20	5205 / 23.15
HGUS26-2	12	3⁵⁄₁₆	5⁷⁄₁₆	4	4⅛	(20) 16d	(8) 16d	4385 / 19.51	8950 / 39.81	3110 / 13.83	6355 / 28.27
LUS28-2	18	3⅛	7	2	4	(6) 16d	(4) 16d	1720 / 7.65	3325 / 14.79	1545 / 6.87	2575 / 11.45
HHUS28-2	14	3⁵⁄₁₆	7⁵⁄₁₆	3	6⅛	(22) 16d	(8) 16d	3765 / 16.75	8940 / 39.77	2675 / 11.90	6345 / 28.22
HGUS28-2	12	3⁵⁄₁₆	7⅜	4	6⅛	(36) 16d	(12) 16d	6070 / 27.00	12980 / 57.74	4310 / 19.17	9215 / 40.99
LUS210-2	18	3⅛	9	2	6	(8) 16d	(6) 16d	2580 / 11.48	4500 / 20.02	2320 / 10.32	3195 / 14.21
HHUS210-2	14	3⁵⁄₁₆	9⅜	3	8	(30) 16d	(10) 16d	4670 / 20.77	9660 / 42.97	4235 / 18.84	7000 / 31.14
HGUS210-2	12	3⁵⁄₁₆	9⅜	4	8⅛	(46) 16d	(16) 16d	6840 / 30.43	14015 / 62.34	4855 / 21.60	10270 / 45.69
Triple 2x Sizes											
HGUS26-3	12	4¹⁵⁄₁₆	5½	4	4⅛	(20) 16d	(8) 16d	4385 / 19.51	8950 / 39.81	3110 / 13.83	6355 / 28.27
HGUS28-3	12	4¹⁵⁄₁₆	7¼	4	6⅛	(36) 16d	(12) 16d	6070 / 27.00	12980 / 57.74	4310 / 19.17	9215 / 40.99
HHUS210-3	14	4¹⁵⁄₁₆	9	3	7¹⁵⁄₁₆	(30) 16d	(10) 16d	4670 / 20.77	9670 / 43.02	4235 / 18.84	6865 / 30.54
HGUS210-3	12	4¹⁵⁄₁₆	9¼	4	8⅛	(46) 16d	(16) 16d	6840 / 30.43	14645 / 65.14	4855 / 21.60	10400 / 46.26
Quadruple 2x Sizes											
HGUS26-4	12	6⁵⁄₁₆	5⁷⁄₁₆	4	4⅛	(20) 16d	(8) 16d	4385 / 19.51	8950 / 39.81	3110 / 13.83	6355 / 28.27
HGUS28-4	12	6⁵⁄₁₆	7⅜	4	6⅛	(36) 16d	(12) 16d	6070 / 27.00	12980 / 57.74	4310 / 19.17	9215 / 40.99
HHUS210-4	14	6⅛	8⅞	3	7¹⁵⁄₁₆	(30) 16d	(10) 16d	4670 / 20.77	10155 / 45.17	4235 / 18.84	7210 / 32.07
HGUS210-4	12	6⁵⁄₁₆	9⅜	4	8⅛	(46) 16d	(16) 16d	6840 / 30.43	14645 / 65.14	4855 / 21.60	10400 / 46.26
HGUS212-4	12	6⁵⁄₁₆	10⅝	4	10⅛	(56) 16d	(20) 16d	7640 / 33.98	14995 / 66.70	5425 / 24.13	10645 / 47.35
HGUS214-4	12	6⁵⁄₁₆	12⅝	4	11⅛	(66) 16d	(22) 16d	10130 / 45.06	16400 / 72.95	7195 / 32.00	11645 / 51.80
4x Sizes											
LUS46	18	3⁵⁄₁₆	4¾	2	3⅞	(4) 16d	(4) 16d	1720 / 7.65	2595 / 11.54	1545 / 6.87	1920 / 8.54
HHUS46	14	3⅝	5¼	3	3¹⁵⁄₁₆	(14) 16d	(6) 16d	2540 / 11.30	7335 / 32.63	2065 / 9.20	5205 / 23.15
HGUS46	12	3⅝	5¼	4	4⅛	(20) 16d	(8) 16d	4385 / 19.51	8950 / 39.81	3110 / 13.83	6355 / 28.27
LUS48	18	3⁵⁄₁₆	6¾	2	3⅞	(6) 16d	(4) 16d	1720 / 7.65	3325 / 14.79	1545 / 6.87	2575 / 11.45
HHUS48	14	3⅝	7⅛	3	6⅛	(22) 16d	(8) 16d	3765 / 16.75	8940 / 39.77	2675 / 11.90	6345 / 28.22
HGUS48	12	3⅝	7⅛	4	6⅛	(36) 16d	(12) 16d	6070 / 27.00	12980 / 57.74	4310 / 19.17	9215 / 40.99
LUS410	18	3⁵⁄₁₆	8¾	2	5⁹⁄₁₆	(8) 16d	(6) 16d	2580 / 11.48	4500 / 20.02	2320 / 10.32	3195 / 14.21
HGUS410	12	3⅝	9	4	8⅛	(46) 16d	(16) 16d	6840 / 30.43	14015 / 62.34	4855 / 21.60	10270 / 45.69
HGUS412	12	3⅝	10⅞	4	10⅛	(56) 16d	(20) 16d	7640 / 33.98	14995 / 66.70	5425 / 24.13	10645 / 47.35
HGUS414	12	3⅝	12⅞	4	11⅛	(66) 16d	(22) 16d	10130 / 45.06	16400 / 72.95	7195 / 32.00	11645 / 51.80

See footnotes on p. 258.

MICRO CITY
ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242



45° Hip End

LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2

BOTTOM CHORD : 2 x 4 SPF#2

WEBS : 2 x 3 SPF#2

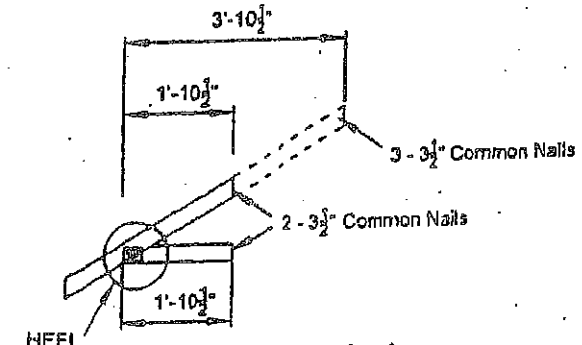
UNLESS OTHERWISE SHOWN

DESIGN LOAD:

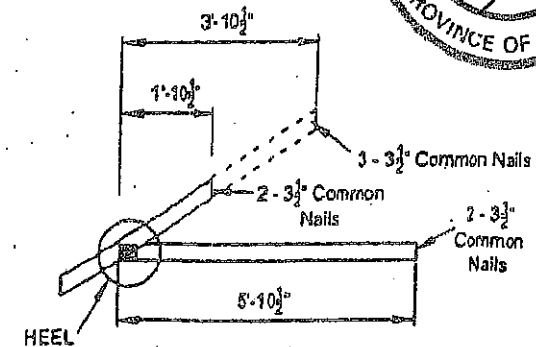
TOP CHORD LIVE LOAD	: 34.8 P.S.F.
TOP CHORD DEAD LOAD	: 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	: 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	: 7.0 P.S.F.
TOTAL LOAD	: 44.8 P.S.F.

DWG NO TAM 3495.14 R1
STRUCTURAL COMPONENT ONLY

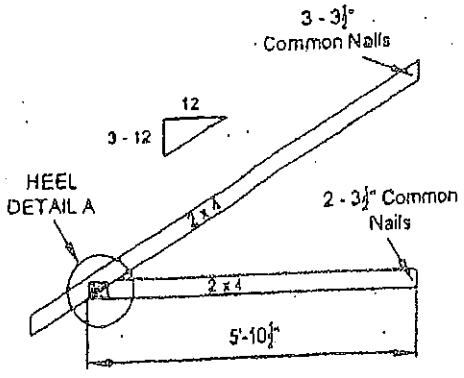
LICENSED PROFESSIONAL ENGINEER
1-20-14
S. KATSOULAKOS
PROVINCE OF ONTARIO



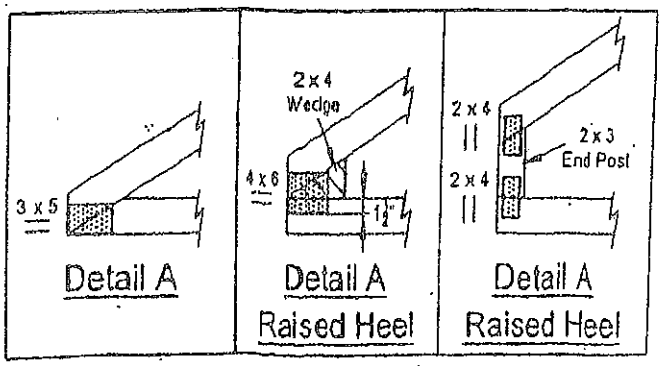
Corner Side Jacks



Corner End Jacks



Common End Jacks



Detail A

Detail A Raised Heel

Detail A Raised Heel

NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

MICRO CITY
ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

LUMBER SPECIFICATION

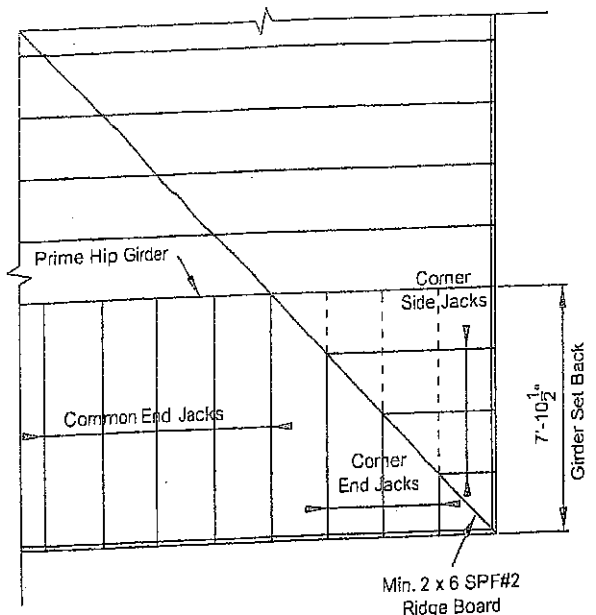
TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

DESIGN LOAD:

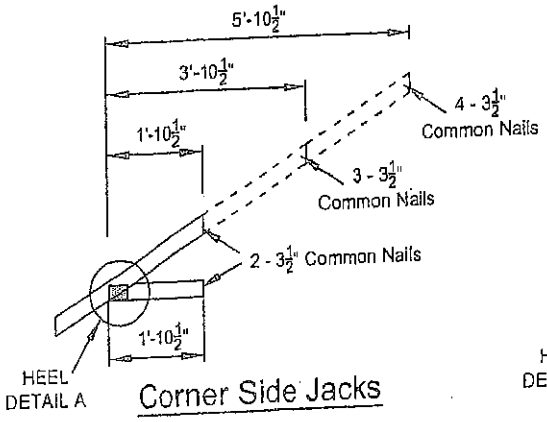
TOP CHORD LIVE LOAD	: 40.5 P.S.F.
TOP CHORD DEAD LOAD	: 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	: 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	: 7.0 P.S.F.
TOTAL LOAD	: 50.5 P.S.F.

DWG NO TAM 3504.1
STRUCTURAL
COMPONENT ONLY

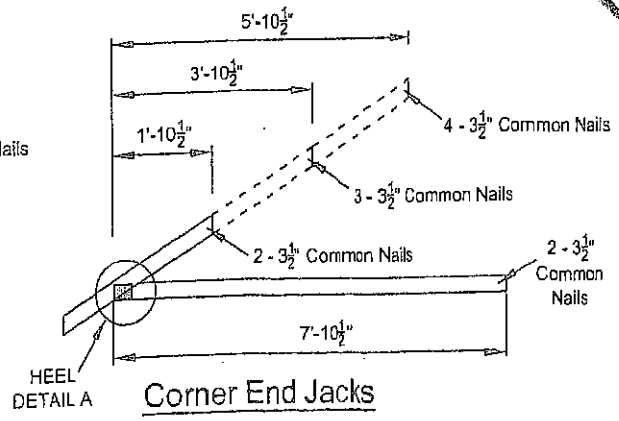
LICENSED PROFESSIONAL ENGINEER
1-2014
S. KATSOULAKOS
PROVINCE OF ONTARIO



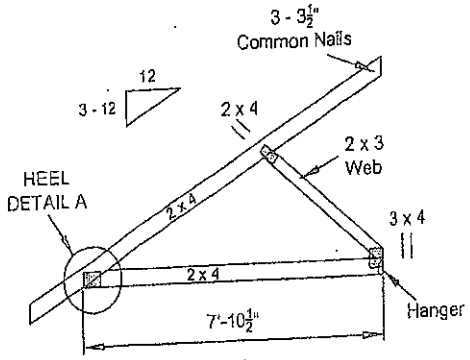
45° Hip End



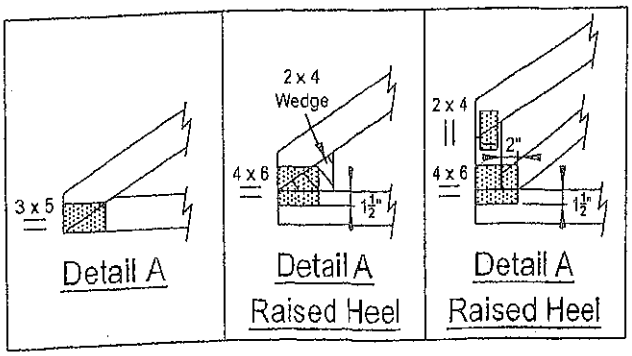
Corner Side Jacks



Corner End Jacks



Common End Jacks



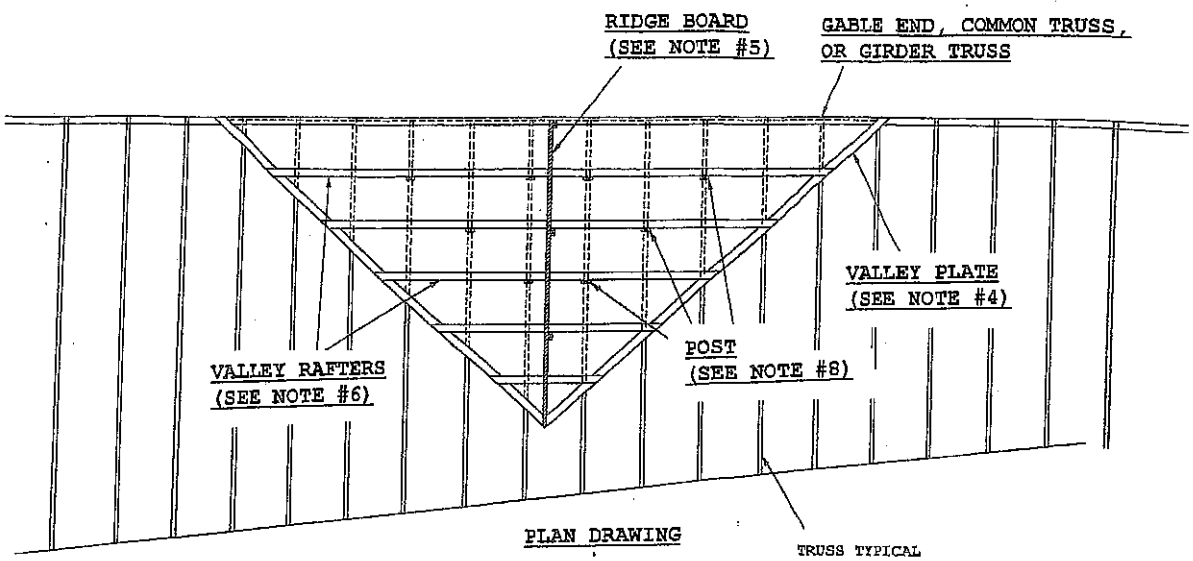
NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

MICRO CITY
ENGINEERING SERVICES INC.

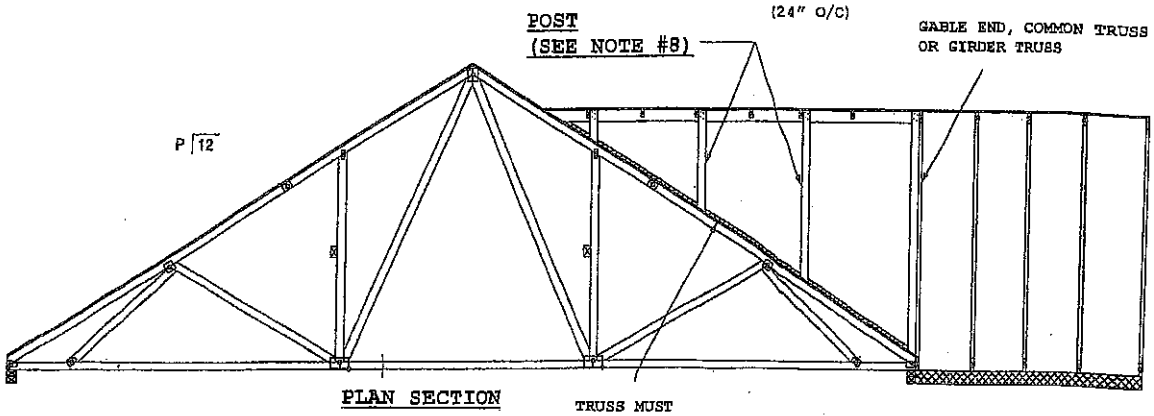
TEL: (519) 287 - 2242

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

CONVENTIONAL VALLEY FRAMING DETAIL



PLAN DRAWING



PLAN SECTION

TRUSS MUST
BE SHEATHED

GENERAL SPECIFICATIONS:

(1) WITH THE BASE TRUSSES ERECTED (INSTALLED), APPLY SHEATHING TOP CHORD OF SUPPORTING (BASE) TRUSSES.
(2) BRACE BOTTOM CHORD AND WEB MEMBERS AS PER PRE-ENGINEERED TRUSS DESIGNS.
(3) DEFINE VALLEY RIDGE BY RUNNING A LEVEL STRING FROM THE INTERSECTING RIDGE OF THE (a) GABLE END, (b) GIRDER TRUSS OR (c) COMMON TRUSS TO THE ROOF SHEATHING.
(4) INSTALL 2 X 6 VALLEY PLATES ON FLAT. FASTEN TO EACH SUPPORTING TRUSS WITH (2) 16d (3.5" X 0.131") NAILS.
(5) SET A 2 X 6 #2 RIDGE BOARD (MAX. 10'-0" RIDGE) OR 2 X 8 #2 SPF RIDGE BOARD (MAX. 20'-0" RIDGE). SUPPORT RIDGE BOARD WITH 2 X 4 POSTS SPACED 48" O/C. BEVEL BOTTOM OF POST TO SET EVENLY ON THE SHEATHING. FASTEN POST TO RIDGE WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST TO ROOF SHEATHING WITH (3) 10d (3" X 0.131") TOE-NAILS.
(6) FRAME VALLEY RAFTERS FROM VALLEY PLATE TO RIDGE BOARD. MAXIMUM RAFTER SPACING IS 24" O/C. FASTEN VALLEY RAFTER TO RIDGE BEAM WITH (3) 16d (3.5" X 0.131") TOE-NAILS. FASTEN VALLEY RAFTER TO VALLEY PLATE WITH (3) 16d (3.5" X 0.131") TOE-NAILS.
(7) SUPPORT THE VALLEY RAFTERS WITH 2 X 4 POSTS AT 48" O/C (OR LESS) ALONG EACH RAFTER. INSTALL POSTS IN A STAGGERED PATTERN AS SHOWN ON PLAN DRAWING. ALIGN POSTS WITH TRUSSES BELOW. FASTEN VALLEY RAFTER TO POST WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST THROUGH SHEATHING TO SUPPORTING TRUSSES WITH (2) 16d (3.5" X 0.131") NAILS.
(8) POSTS SHALL BE 2 X 4 #2 SPF OR BETTER. POSTS EXCEEDING 75" IN HEIGHT SHALL BE INCREASED TO 4 X 4 #2 SPF, OR BETTER, OR BE PRE-ASSEMBLED TWO (2) PLY 2 X 4 #2 SPF OR BETTER FASTENED TOGETHER WITH 2 ROWS OF 10d (3" X 0.131") NAILS AT 6" O/C.
(9) MAINTAIN A MINIMUM 3/4" LUMBER EDGE DISTANCE WHEN NAILING. NAIL SPACING SHOULD APPROXIMATE A MINIMUM 1-3/4" O/C OR MORE UNLESS NOTED OTHERWISE. ALL CONSTRUCTION TO CONFORM TO ONTARIO BUILDING CODE (CURRENT ADDITION) AT ALL TIMES.

NOTES:

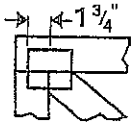
(10) 48" O/C (MAXIMUM POST SPACING.
(11) ROOF LIVE LOAD = 34.8 PSF (MAX.)
(12) ROOF DEAD LOAD = 10.0 PSF (MAX.)
(13) PART 9 APPLICATION ONLY (ONTARIO BUILDING CODE)
(14) PART 4 APPLICATION ONLY (ONTARIO BUILDING CODE) WITH APPROVED REVIEW BY LICENSED PROFESSIONAL ENGINEER.
(15) BASE TRUSS SPACING (24" O/C MAX.)
(16) ALL PRE-ENGINEERED BASE TRUSS COMPONENTS TO BE SEALED BY LICENSED PROFESSIONAL ENGINEER AND THIS DETAIL TO BE VERIFIED AND APPROVED BY SAME WHEN RIDGE BOARD LENGTH EXCEEDS 12'-0".
(17) ALL BASE TRUSSES: P = 4 (4/12) - MINIMUM.
(18) ALL VALLEY RAFTERS: P = 4 (4/12) - MINIMUM.

LICENSED PROFESSIONAL ENGINEER
3-574
S. KATSOULAKOS
PROVINCE OF ONTARIO

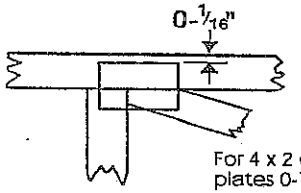
DWG NO TAM6305.14
STRUCTURAL
COMPONENT ONLY

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

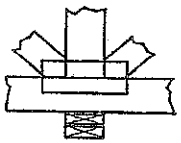
4 X 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

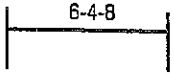
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses

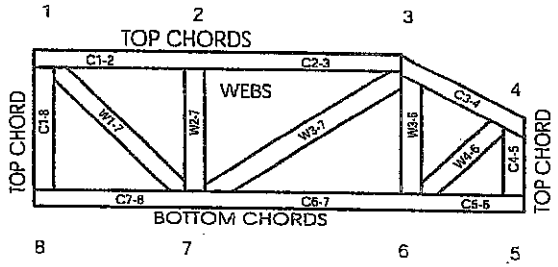
DSB-89: Design Standard for Bracing.

BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

· CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

MiTek

POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MII-7473C rev. 10-'08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

Micro City Engineering Services Inc.
(BCIN: 26064; FIRM BCIN: 29991)
RR #1, Po Box 61
Glencoe, Ontario
N0L 1M0
(519) 287 - 2242; Fax: (519) 287 - 5750 (Call)

LICENSED PROFESSIONAL ENGINEER
1-2014
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO T4M 5474J14
STRUCTURAL
COMPONENT ONLY

Responsibilities:

Micro City Engineering Services is responsible for the design of trusses as individual components.

It is the responsibilities of others to ascertain that the design loads utilized on this (these) drawing(s) meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdiction over such decisions.

All dimensions are to be verified by the owner, contractor, architect, or other authority having input over such decisions prior to truss component manufacture. At no time shall Micro City Engineering Services Inc. or its employees be responsible for dimension errors.

Micro City Engineering Services Inc. bears no responsibility for the erection of any truss components. Persons erecting truss components are cautioned to seek professional advice regarding temporary and permanent bracing systems and to be **totally** familiar with all aspects of truss erection prior to proceeding on **any** truss component erection job. Any bracing shown on Micro City Engineering Services Inc. or Tamarack Roof Trusses Inc. sealed or unsealed truss component drawings is specified for the single truss component in question and is identified as an integral part of the design for that particular truss component but is **not** meant to represent the only required bracing for that particular truss component when installed as a component in a series of truss components in a roof truss system.

It is the truss manufacturer's responsibility to ensure that trusses are manufactured in accordance with Micro City Engineering Services Inc. specifications outlined below:

SPECIFICATIONS:

Truss components sealed by Micro City Engineering Services Inc. must conform to the relevant sections of the current Building Code of Ontario and Canada (Part 4 or Part 9) or the current Farm Building Code of Canada in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the Truss Plate Institute of Canada (TPIC). All unit lumber and nailing stresses identified on truss component design drawings and/or used in the design of individual truss components shall conform to the current CSA Wood Design standard identified in the current Building Code and TPIC Design Standards.

The lumber used to manufacture any truss component is to conform to the specified size and grade identified on the truss drawing.

The lumber used in the manufacture of any truss component is not to exceed 19% during its service use unless specifically noted on the truss drawing.

The lumber used in the manufacture of any truss component is not to be treated with any chemicals during its service life unless specifically noted on the truss drawing.

Connector plates shall be applied to both faces of the truss component at each joint and shall be positioned exactly as specified.

The top chord of any truss component is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the sealed truss component drawing but not exceeding 24" o/c (Part 9 design) and not exceeding 48" o/c (Part 4 or Agricultural design).

When a truss component is to be installed with no rigid ceiling attached directly to the bottom chord, then the bottom chord is to be laterally braced at intervals not exceeding 3m (or 10'-0").

All sealed or unsealed truss component drawings provided by Micro City Engineering Services Inc. Or Tamarack Roof Trusses Inc. should be read in conjunction with the following:

Warning-Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473C rev 10-'08 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer - not the truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component Safety Information available from the Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA, 22314.