ASPHALT SHINGLES
12"FINISH O.H.
R.T.M.C
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:

HGUS26-2 -(XX)
LJS26DS -(V)

8/12 PITCHES (TYP.)
UNLESS NOTED

DENOTES:

H -2'-0-0 HIGHER PLATE

-1'-6"-0 RAISED FASCIA

ALL B- 2-2X10 BEAMS (FLUSH)

18" HIGHER PLATE AREA

HIGHGROVE-12
EL:2 - UNIT1

285474

HIGHGROVE-4
EL:2- UNIT2

285475

HIGHGROVE-2
EL:2(REV) - UNIT3

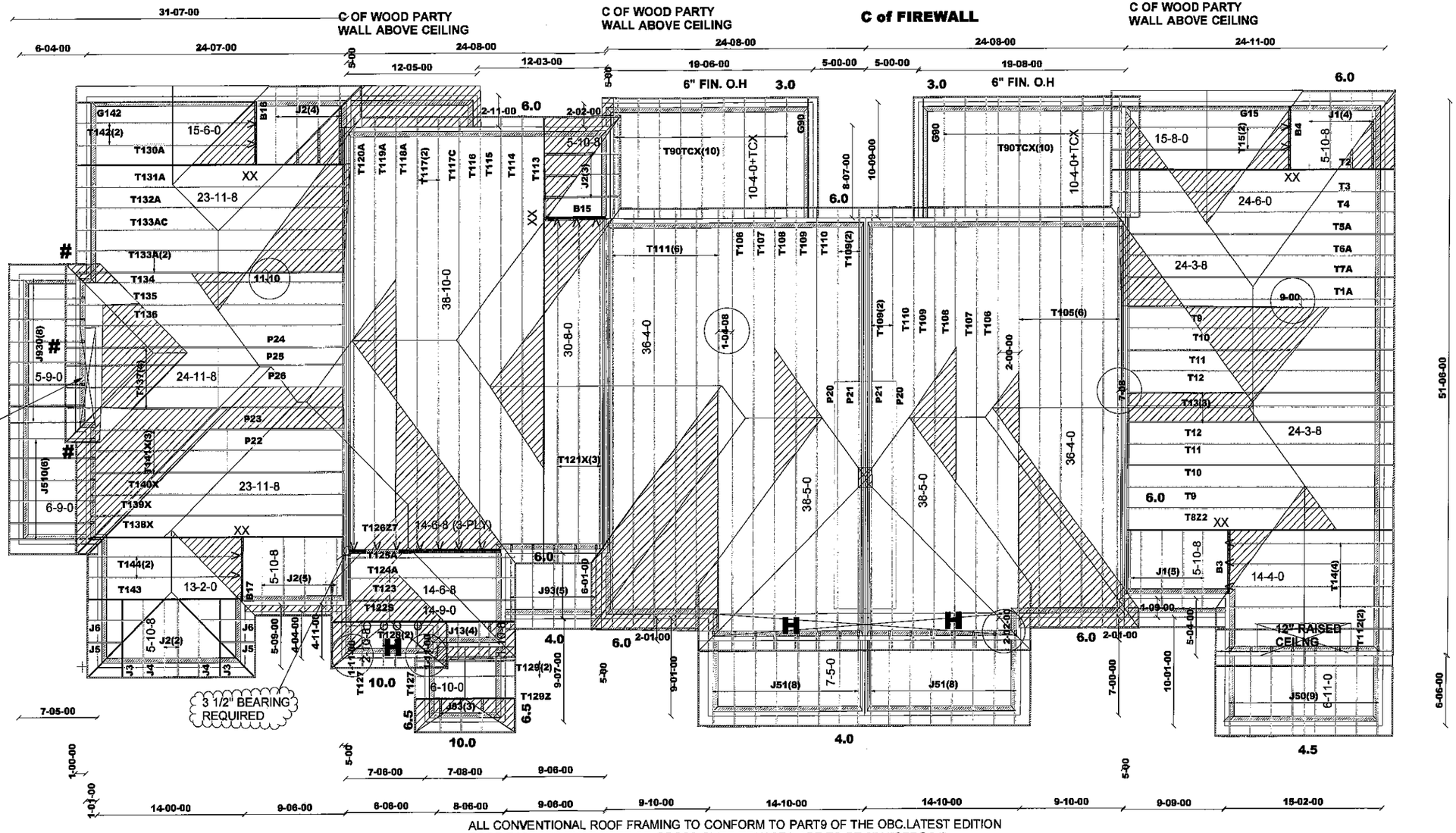
285477

HIGHGROVE-2
EL:2 - UNIT4

285478

HIGHGROVE-5E
EL:2(REV) - UNIT5

285479



T-170534

DENOTES:
CONVENTIONAL FRAMING

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION
ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"o.c.
WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.
POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE
BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN LOADS:

SNOW LOAD 25.6 PSF
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

m10,986

TAMARACK LUMBER INC. ALPA LUMBER GROUP

Job Track:	43954
Layout ID:	285473
Plan Log:	93446

Builder / Location: **GREENPARK HOMES / WATERDOWN**
Model / Elevation: **BLOCK201 / UNIT1TO5 (REVISED)**
Mitek ver 7.5.0
Project: **RUSSEL GARDENS PHASE 2**
Date: 4/17/2018 Designer: sonny

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

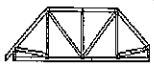
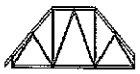
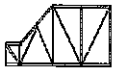
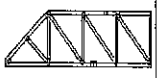
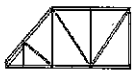
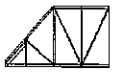
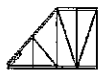
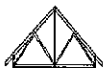
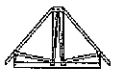
Delivery Shiplist

DATE	09/01/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 285474 LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH 2 SUB-BUILDER:
MODEL: HIGHGROVE 12 ELEVATION: 2- UNIT1

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T130A HIP GIRDER	8.00 0.00	23-11-08	05-03-13	2 X 6	2 X 6	01-03-08 00-00-00	01-04-13 01-06-08	140.50 86.67		
	1	T131A HIP	8.00 0.00	23-11-08	06-07-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	100.98 65.83		
	1	T132A HIP	8.00 0.00	23-11-08	07-11-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	107.29 68.83		
	2	T133A COMMON	8.00 0.00	23-11-08	09-05-08	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	203.14 127.00		
	1	T133AC COMMON	8.00 0.00	23-11-08	09-03-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	101.39 63.50		
	1	T134 ROOF	8.00 0.00	24-11-08	08-01-09	2 X 4	2 X 4	00-00-00 00-00-00	00-03-08 08-01-09	122.33 77.17		
	1	T135 ROOF	8.00 0.00	24-11-08	09-01-09	2 X 4	2 X 4	01-03-08 00-00-00	02-10-13 09-01-09	134.44 85.00		
	1	T136 ROOF	8.00 0.00	24-11-08	10-01-09	2 X 4	2 X 4	01-03-08 00-00-00	02-10-13 10-01-09	143.42 89.17		
	4	T137 ROOF	8.00 0.00	24-11-08	09-05-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 09-05-13	564.72 356.68		
	1	T138X HALF HIP	8.00 0.00	23-11-08	05-03-13	2 X 6	2 X 6	00-00-00 00-00-00	00-08-02 05-03-13	137.49 82.67		
	1	T139X HALF HIP	8.00 0.00	23-11-08	06-07-13	2 X 4	2 X 4	00-00-00 00-00-00	00-08-02 06-07-13	103.32 65.17		
	1	T140X HALF HIP	8.00 0.00	23-11-08	07-11-13	2 X 4	2 X 4	00-00-00 00-00-00	00-08-02 07-11-13	111.52 70.00		
	3	T141X HALF HIP	8.00 0.00	23-11-08	09-03-13	2 X 4	2 X 4	00-00-00 00-00-00	00-08-02 09-03-13	366.33 224.49		
	2	T142 COMMON	8.00 0.00	15-06-00	06-06-13	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	132.28 84.00		
	1	G142 GABLE	8.00 0.00	15-06-00	06-06-13	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	66.17 42.00		
	1	T143 HIP GIRDER	8.00 0.00	13-02-00	05-03-13	2 X 4	2 X 6	01-03-08 01-03-08	01-04-13 01-04-13	71.46 45.33		
	2	T144 COMMON	8.00 0.00	13-02-00	05-09-08	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	114.76 74.00		
	1	P22 PIGGYBACK	8.00 0.00	10-03-01	01-02-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 01-02-08	26.43 17.00		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

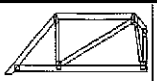
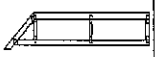
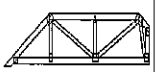
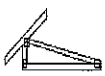
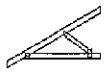
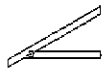
Delivery Shiplist

DATE	09/01/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 285474 LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH 2 SUB-BUILDER:
MODEL: HIGHGROVE 12 ELEVATION: 2- UNIT1

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	1	P23 PIGGYBACK	8.00 0.00	10-03-01	02-06-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 01-11-03	35.54 23.50		
	1	P24 PIGGYBACK	8.00 0.00	11-01-01	01-06-04	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 01-06-04	29.30 18.33		
	1	P25 PIGGYBACK	8.00 0.00	11-01-01	02-04-03	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 01-09-03	39.51 26.33		
	1	P26 PIGGYBACK	8.00 0.00	11-01-01	01-02-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 01-02-08	28.33 18.33		
	11	J2 JACK-OPEN	8.00 0.00	05-10-08	05-03-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-03-13	196.13 117.37		
	2	J3 JACK-OPEN	8.00 0.00	05-10-08	02-07-02	2 X 4	2 X 4	01-03-08 -04-01-01	01-04-13 00-03-08	33.68 20.66		
	2	J4 JACK-OPEN	8.00 0.00	05-10-08	03-11-02	2 X 4	2 X 4	01-03-08 -02-01-01	01-04-13 00-03-08	39.16 24.66		
	2	J5 JACK-OPEN	8.00 0.00	01-10-08	02-07-02	2 X 4	2 X 4	01-03-08 -00-01-01	01-04-13 00-03-08	18.34 11.34		
	2	J6 JACK-OPEN	8.00 0.00	01-10-08	03-11-02	2 X 4	2 X 4	01-03-08 01-10-15	01-04-13 00-05-03	23.82 15.34		
	6	J510 JACK	4.00 0.00	06-09-00	02-06-15	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-06-15	117.24 73.02		
	8	J930 JACK	4.00 0.00	05-09-00	02-02-15	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-02-15	121.04 74.64		

TOTAL # TRUSS= **63.00** TOTAL BFT OF ALL TRUSSES= **2148.03** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3430.06** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
4	Hangers	LJS26DS	
2	Hangers	HGUS26-2	

TOTAL # ITEMS= **6.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

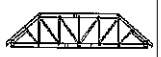
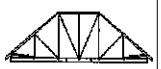
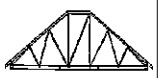
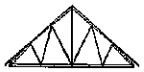
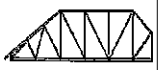
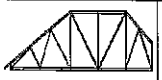
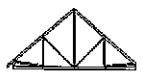
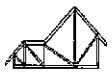
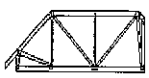
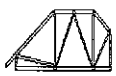
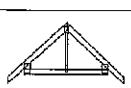
Delivery Shiplist

DATE	08/23/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 285475 LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH 2 SUB-BUILDER:
MODEL: HIGHGROVE4 ELEVATION: 2-UNIT2

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	1	T113 HIP GIRDER	6.00 0.00	38-10-00	05-03-13	2 X 6	2 X 6	01-03-08 01-03-08	01-02-00 01-02-00	251.22 154.00		
	1	T114 HIP	6.00 0.00	38-10-00	06-07-13	2 X 4	2 X 6	01-03-08 01-03-08	01-02-00 01-02-00	184.92 113.00		
	1	T115 HIP	6.00 0.00	38-10-00	07-11-13	2 X 4	2 X 6	01-03-08 01-03-08	01-02-00 01-02-00	185.04 112.67		
	1	T116 HIP	6.00 0.00	38-10-00	09-03-13	2 X 4	2 X 6	01-03-08 01-03-08	01-02-00 01-02-00	189.73 115.67		
	2	T117 COMMON	6.00 0.00	38-10-00	10-10-08	2 X 4	2 X 6	01-03-08 01-03-08	01-02-00 01-02-00	376.36 230.00		
	1	T117C COMMON	6.00 0.00	38-10-00	10-07-13	2 X 4	2 X 6	01-03-08 01-03-08	01-02-00 01-02-00	187.96 115.00		
	1	T118A ROOF	6.00 0.00	38-10-00	08-00-08	2 X 4	2 X 6	01-03-08 00-00-00	01-02-00 04-06-13	193.99 118.17		
	1	T119A ROOF	6.00 0.00	38-10-00	09-04-08	2 X 4	2 X 6	01-03-08 00-00-00	01-02-00 04-06-13	203.82 124.83		
	1	T120A ROOF	6.00 0.00	38-10-00	10-08-08	2 X 4	2 X 6	01-03-08 00-00-00	01-02-00 04-06-13	214.74 131.50		
	3	T121X COMMON	6.00 0.00	30-08-00	08-08-12	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 00-11-08	353.34 219.99		
	1	T122S ROOF	8.00 0.00	14-09-00	06-00-07	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 03-04-13	65.51 41.84		
	1	T123A HIP	8.00 0.00	14-06-08	03-11-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 03-06-08	61.84 39.34		
	1	T124A HIP	8.00 0.00	14-06-08	05-03-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 03-06-08	66.41 42.00		
	1	T125A HIP	8.00 0.00	14-06-08	06-07-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 03-06-08	70.54 46.17		
	1 3 Ply	T126 HIP GIRDER	8.00 0.00	14-06-08	06-11-02	2 X 4	2 X 6	00-00-00 00-00-00	01-04-13 03-06-08	291.93 191.01		
	2	T127 HALF HIP	10.00 0.00	02-10-08	02-04-13	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 04-04-13	38.22 27.34		
	2	T128 HALF HIP	10.00 0.00	02-10-08	03-08-13	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 05-08-13	37.94 25.66		
	2	T129 COMMON	6.50 0.00	06-10-00	02-05-10	2 X 4	2 X 4	01-03-08 01-03-08	00-07-07 00-07-07	46.18 30.00		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

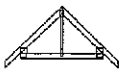
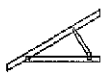
Delivery Shiplist

DATE	08/23/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 285475 LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH2 SUB-BUILDER:
MODEL: HIGHGROVE4 ELEVATION: 2-UNIT2

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T129Z COMMON	6.50 0.00	06-10-00	02-05-10	2 X 4	2 X 4	01-03-08 01-03-08	00-07-07 00-07-07	23.09 15.00		
	3	J2 JACK-OPEN	8.00 0.00	05-10-08	05-03-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-03-13	53.49 32.01		
	4	J13 JACK-OPEN	8.00 0.00	01-10-08	02-07-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 02-07-13	31.16 18.68		
	3	J53 JACK-OPEN	10.00 0.00	01-08-11	02-05-10	2 X 4	2 X 4	01-03-08 00-00-00	01-00-06 02-05-10	21.45 15.51		
	5	J93 MONOPITCH	4.00 0.00	06-05-00	02-05-10	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-05-10	90.80 58.35		

TOTAL # TRUSS= **42** TOTAL BFT OF ALL TRUSSES= **2017.74** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3239.68** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	HGUS26-2	
10	Hangers	LJS26DS	
4	Hangers	LUS24	

TOTAL # ITEMS= **15**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

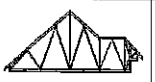
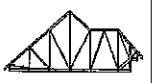
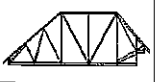
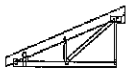
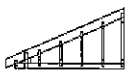
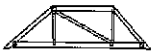
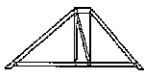
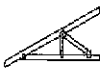
Delivery Shiplist

DATE	08/23/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 285477 LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH 2 SUB-BUILDER:
MODEL: HIGHGROVE2 ELEVATION: 2 (REV)-UNIT3

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	6	T111 ROOF	6.00 0.00	36-04-00	08-08-12	2 X 4	2 X 6	01-03-08 01-03-08	01-02-00 01-02-00	1058.52 664.98		
	1	T106 ROOF	6.00 0.00	38-05-00	09-04-13	2 X 4	2 X 6	01-03-08 01-03-08	01-02-00 01-04-13	210.94 130.00		
	1	T107 ROOF	6.00 0.00	38-05-00	10-03-00	2 X 4	2 X 6	01-03-08 01-03-08	01-02-00 01-04-13	200.19 125.33		
	1	T108 ROOF	6.00 0.00	38-05-00	10-03-00	2 X 4	2 X 6	01-03-08 01-03-08	01-02-00 01-04-13	203.82 125.67		
	3	T109 HIP	6.00 0.00	38-05-00	08-06-13	2 X 4	2 X 6	01-03-08 01-03-08	01-02-00 01-04-13	624.12 381.99		
	1	T110 HIP	6.00 0.00	38-05-00	09-10-13	2 X 4	2 X 6	01-03-08 01-03-08	01-02-00 01-04-13	221.07 137.50		
	10	T90TCX MONOPITCH	3.00 0.00	10-04-00	03-05-09	2 X 6	2 X 4	00-09-08 00-00-00	00-08-04 03-05-09	449.90 303.30		
	1	G90 MONOPITCH	3.00 0.00	10-04-00	03-03-04	2 X 6	2 X 4	00-09-08 00-00-00	00-08-04 03-03-04	40.90 26.67		
	1	P20 PIGGYBACK	6.00 0.00	14-02-02	02-06-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-03 00-04-07	44.32 27.50		
	1	P21 PIGGYBACK	6.00 0.00	14-02-02	03-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-03 00-04-07	45.00 28.67		
	8	J51 JACK TRUSS	4.00 0.00	07-05-00	02-09-10	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-09-10	177.36 116.00		

TOTAL # TRUSS= **34.00** TOTAL BFT OF ALL TRUSSES= **2067.61** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3276.14** LBS.

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

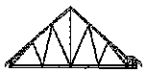
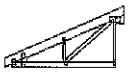
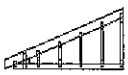
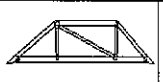
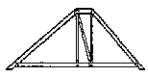
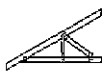
Delivery Shiplist

DATE	08/23/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 285478 LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH 2 SUB-BUILDER:
MODEL: HIGHGROVE2 ELEVATION: 2-UNIT4

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	6	T105 ROOF	6.00 0.00	36-04-00	09-09-12	2 X 4	2 X 6	01-03-08 01-03-08	01-02-00 01-02-00	1043.58 655.02		
	1	T106 ROOF	6.00 0.00	38-05-00	09-04-13	2 X 4	2 X 6	01-03-08 01-03-08	01-02-00 01-04-13	210.94 130.00		
	1	T107 ROOF	6.00 0.00	38-05-00	10-03-00	2 X 4	2 X 6	01-03-08 01-03-08	01-02-00 01-04-13	200.19 125.33		
	1	T108 ROOF	6.00 0.00	38-05-00	10-03-00	2 X 4	2 X 6	01-03-08 01-03-08	01-02-00 01-04-13	203.82 125.67		
	3	T109 HIP	6.00 0.00	38-05-00	08-06-13	2 X 4	2 X 6	01-03-08 01-03-08	01-02-00 01-04-13	624.12 381.99		
	1	T110 HIP	6.00 0.00	38-05-00	09-10-13	2 X 4	2 X 6	01-03-08 01-03-08	01-02-00 01-04-13	221.07 137.50		
	10	T90TCX MONOPITCH	3.00 0.00	10-04-00	03-05-09	2 X 6	2 X 4	00-09-08 00-00-00	00-08-04 03-05-09	449.90 303.30		
	1	G90 MONOPITCH	3.00 0.00	10-04-00	03-03-04	2 X 6	2 X 4	00-09-08 00-00-00	00-08-04 03-03-04	40.90 26.67		
	1	P20 PIGGYBACK	6.00 0.00	14-02-02	02-06-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-03 00-04-07	44.32 27.50		
	1	P21 PIGGYBACK	6.00 0.00	14-02-02	03-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-03 00-04-07	45.00 28.67		
	8	J51 JACK TRUSS	4.00 0.00	07-05-00	02-09-10	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-09-10	177.36 116.00		

TOTAL # TRUSS= **34.00** TOTAL BFT OF ALL TRUSSES= **2057.65** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3261.20** LBS.

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

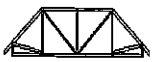
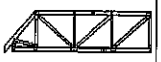
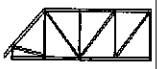
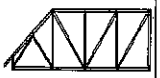
Delivery Shiplist

DATE	04/17/18
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 285479 LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH 2 SUB-BUILDER:
MODEL: HIGHGROVE 5E ELEVATION: 2(REV)- UNIT5

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T1A COMMON	8.00 0.00	24-03-08	09-06-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	102.41 64.00		
	1	T2 HIP GIRDER	8.00 0.00	24-06-00	04-01-04	2 X 6	2 X 6	01-03-08 01-01-00	01-04-13 01-04-13	145.29 89.67		
	1	T3 HIP	8.00 0.00	24-06-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	97.09 60.83		
	1	T4 HIP	8.00 0.00	24-06-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	105.79 67.00		
	1	T5A HIP	8.00 0.00	24-03-08	07-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	104.32 67.00		
	1	T6A HIP	8.00 0.00	24-03-08	08-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	108.82 69.50		
	1	T7A HIP	8.00 0.00	24-03-08	09-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-08	114.67 72.50		
	1	T8Z2 HALF HIP	8.00 0.00	24-03-08	04-01-04	2 X 6	2 X 6	01-03-08 00-00-00	01-04-13 04-01-04	138.05 86.67		
	2	T9 HALF HIP	8.00 0.00	24-03-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-01-04	197.10 123.66		
	2	T10 HALF HIP	8.00 0.00	24-03-08	06-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 06-01-04	212.66 133.34		
	2	T11 HALF HIP	8.00 0.00	24-03-08	07-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 07-01-04	225.56 142.68		
	2	T12 HALF HIP	8.00 0.00	24-03-08	08-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 08-01-04	234.64 146.34		
	3	T13 HALF HIP	8.00 0.00	24-03-08	09-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 09-01-04	368.64 227.49		
	2	T112 ROOF	8.00 0.00	14-04-00	06-02-02	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	135.80 89.00		
	4	T14 COMMON	8.00 0.00	14-04-00	06-02-02	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	247.04 156.00		
	2	T15 COMMON	8.00 0.00	15-08-00	06-07-08	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	133.52 84.00		
	1	G15 COMMON	8.00 0.00	15-08-00	06-07-08	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	69.72 45.00		
	9	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	151.11 96.03		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	04/17/18
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 285479 LOCATION:
BUILDER: GREENPARK - RUSSELL GARDENS PH2 SUB-BUILDER:
MODEL: HIGHGROVE 5E ELEVATION: 2(REV)- UNIT5

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	9	J50 JACK	4.50 0.00	06-11-00	02-11-12	2 X 4	2 X 4	01-03-08 00-00-00	00-04-10 02-11-12	184.86 114.03		

TOTAL # TRUSS= **46.00** TOTAL BFT OF ALL TRUSSES= **1934.74** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3077.09** LBS.

HARDWARE

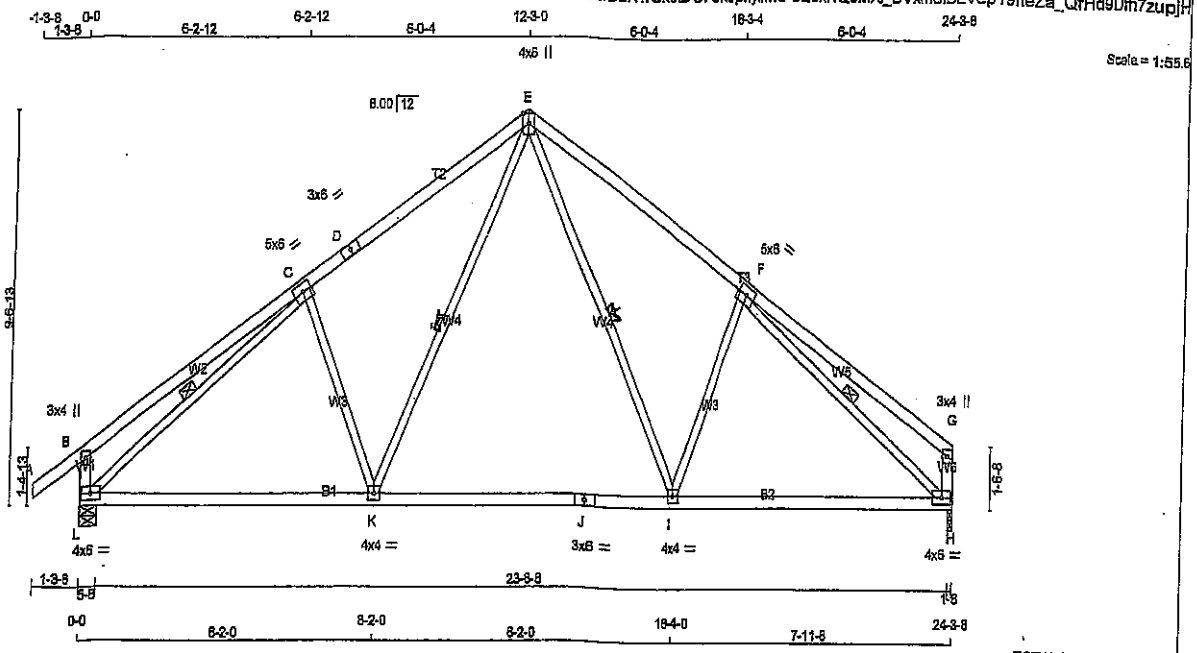
QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
5	Hangers	LJS26DS	

TOTAL # ITEMS= **7.00**

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285994	T1A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Dec 12 2017 MiTek Industries, Inc. Tue Jan 16 10:45:48 2018 Page 1
ID:w3K2IB2K4fGk9lDO7SkJphyfMC-oE6xRQ0MXt_DVxmSIBEVep19heZa_QfHd9Dfh7zupjH



TOTAL WEIGHT = 102 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESGR
A - D	2x4	DRY	No.2 SPF
D - E	2x4	DRY	No.2 SPF
E - G	2x4	DRY	No.2 SPF
L - B	2x4	DRY	No.2 SPF
H - G	2x4	DRY	No.2 SPF
L - J	2x4	DRY	No.2 SPF
J - H	2x4	DRY	No.2 SPF
ALL WEBS EXCEPT	2x3	DRY	No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMV+p	MT20	3.0	4.0	
C	TMWW-t	MT20	5.0	6.0	
D	TS-t	MT20	3.0	6.0	
E	TTWW+p	MT20	4.0	6.0	Edge
F	TMWW-t	MT20	5.0	6.0	
G	TMV+p	MT20	3.0	4.0	
H	BMVW1-t	MT20	4.0	6.0	
I	BMWW-t	MT20	4.0	4.0	
J	BS-t	MT20	3.0	6.0	
K	BMWW-t	MT20	4.0	4.0	
L	BMVW1-t	MT20	4.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
L	1479	0	1479	0	0	5-8	5-8
H	1393	0	1393	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1198	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0
H	1119	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.96 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-L, F-H. E-K, E-J

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00
B-C	0 / 34	-84.3	-84.3	0.52 (1)	10.00
C-D	-1409 / 0	-84.3	-84.3	0.45 (1)	4.96
D-E	-1409 / 0	-84.3	-84.3	0.45 (1)	4.96
E-F	-1388 / 0	-84.3	-84.3	0.44 (1)	5.01
F-G	0 / 34	-84.3	-84.3	0.50 (1)	10.00
L-B	-311 / 0	0.0	0.0	0.03 (1)	7.81
H-G	-186 / 0	0.0	0.0	0.02 (1)	7.81
L-K	0 / 1275	-28.0	-28.0	0.58 (2)	10.00
K-J	0 / 889	-28.0	-28.0	0.54 (2)	10.00
J-I	0 / 889	-28.0	-28.0	0.54 (2)	10.00
I-H	0 / 1245	-28.0	-28.0	0.56 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-K	-352 / 44	0.19 (1)
K-E	0 / 632	0.14 (1)
E-I	0 / 599	0.13 (1)
I-F	-316 / 58	0.17 (1)
L-C	-1689 / 0	0.71 (1)
F-H	-1653 / 0	0.67 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.29")

CSI: TC=0.52/1.00 (B-C:1) , BC=0.56/1.00 (K-L:2) , WB=0.71/1.00 (C-L:1) , SSI=0.21/1.00 (C-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (L) (INPUT = 0.90)
JSI METAL= 0.43 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
1-7618
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM3984-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285473	T2	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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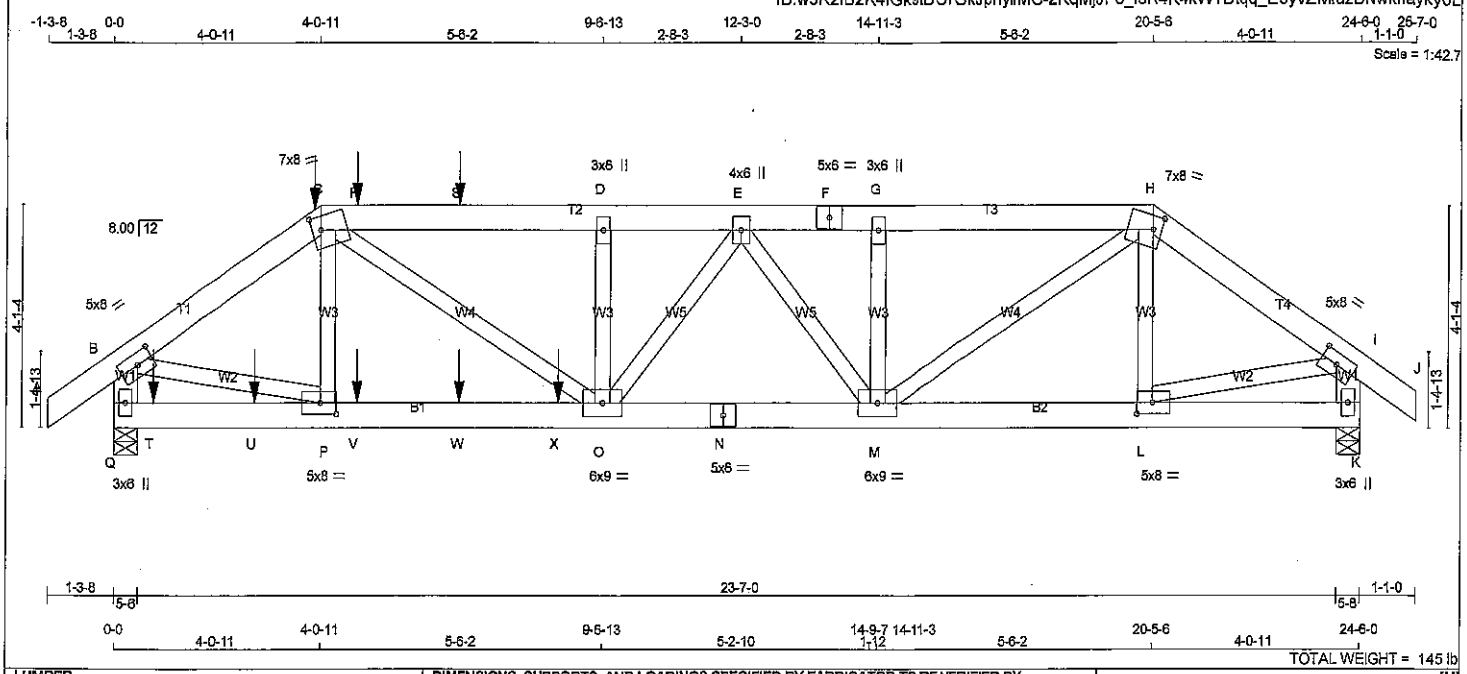

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
Q - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
Q - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.50	3.75
C	TTWW-m	MT20	7.0	8.0	3.00	2.00
D	TMW+w	MT20	3.0	6.0		
E	TMWW+t	MT20	4.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMW+w	MT20	3.0	6.0		
H	TTWW-m	MT20	7.0	8.0	3.00	2.00
I	TMVW-t	MT20	5.0	8.0	2.50	3.75
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	8.0	2.50	3.75
M	BMWWW-t	MT20	6.0	9.0		
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	6.0	9.0		
P	BMWW-t	MT20	5.0	8.0	2.50	3.75
Q	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 244.5 lbs FACTORED DOWN AT 4-0-11, AND 113.4 lbs FACTORED DOWN AT 4-9-4, AND 101.4 lbs FACTORED DOWN AT 6-9-4 ON TOP CHORD, AND 75.5 lbs FACTORED DOWN AT 9-4, 69.9 lbs FACTORED DOWN AT 2-9-4, 69.9 lbs FACTORED DOWN AT 4-9-4, AND 69.8 lbs FACTORED DOWN AT 6-9-4, AND 1159.2 lbs FACTORED DOWN AT 9-9-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
Q	2749	0	2749	0	0	5-8	5-8
K	2004	0	2004	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	2239	1272 / 0	490 / 0	0 / 0	0 / 0	476 / 0	0 / 0
K	1624	936 / 0	347 / 0	0 / 0	0 / 0	341 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 33	-84.3	-84.3	0.07 (1)	10.00		P- C	-122 / 260	0.05 (3)
B- C	-3286 / 0	-84.3	-84.3	0.19 (1)	4.50		C- O	0 / 2143	0.38 (1)
C- R	-4513 / 0	-84.3	-84.3	0.44 (1)	3.66		O- D	-528 / 0	0.09 (1)
R- S	-4513 / 0	-84.3	-84.3	0.44 (1)	3.66		M- G	-441 / 0	0.08 (1)
S- D	-4513 / 0	-84.3	-84.3	0.44 (1)	3.66		M- H	0 / 2033	0.36 (1)
D- E	-4513 / 0	-84.3	-84.3	0.27 (1)	3.85		L- H	-219 / 75	0.04 (1)
E- F	-3803 / 0	-84.3	-84.3	0.20 (1)	4.33		B- P	0 / 2788	0.49 (1)
F- G	-3603 / 0	-84.3	-84.3	0.20 (1)	4.33		L- I	0 / 1956	0.35 (1)
G- H	-3603 / 0	-84.3	-84.3	0.27 (1)	4.25		O- E	0 / 739	0.13 (1)
H- I	-2305 / 0	-84.3	-84.3	0.16 (1)	5.22		E- M	-796 / 0	0.19 (1)
I- J	0 / 25	-84.3	-84.3	0.05 (1)	10.00				
Q- B	-2706 / 0	0.0	0.0	0.19 (1)	6.30				
K- I	-1967 / 0	0.0	0.0	0.14 (1)	7.15				
Q- T	0 / 0	-28.0	-28.0	0.23 (2)	10.00				
T- U	0 / 0	-28.0	-28.0	0.23 (2)	10.00				
U- P	0 / 0	-28.0	-28.0	0.23 (2)	10.00				
P- V	0 / 2722	-28.0	-28.0	0.77 (1)	10.00				
V- W	0 / 2722	-28.0	-28.0	0.77 (1)	10.00				
W- X	0 / 2722	-28.0	-28.0	0.77 (1)	10.00				
X- O	0 / 2722	-28.0	-28.0	0.77 (1)	10.00				
O- N	0 / 4075	-28.0	-28.0	0.79 (1)	10.00				
N- M	0 / 4075	-28.0	-28.0	0.79 (1)	10.00				
M- L	0 / 1904	-28.0	-28.0	0.33 (1)	10.00				
L- K	0 / 0	-28.0	-28.0	0.10 (2)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE
C	4-0-11	-23	-25	—	FRONT	VERT	DEAD
C	4-0-11	-222	-222	—	FRONT	VERT	SNOW
R	4-9-4	-113	-113	—	FRONT	VERT	TOTAL
S	6-9-4	-101	-101	—	FRONT	VERT	TOTAL
T	9-4	-43	-78	—	FRONT	VERT	TOTAL
U	2-8-4	-40	-70	—	FRONT	VERT	TOTAL
V	4-9-4	-40	-70	—	FRONT	VERT	TOTAL
W	6-9-4	-40	-70	—	FRONT	VERT	TOTAL
X	8-8-8	-1159	-1159	—	FRONT	VERT	TOTAL

[M]

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.82")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.82")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.23")

CSI: TC=0.44 (C-D:1) , BC=0.79 (M-O:1) , WB=0.49 (B-P:1) , SSI=0.72 (O-P:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.65 (N) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER 8240 S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM 4192-17
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285473	T3	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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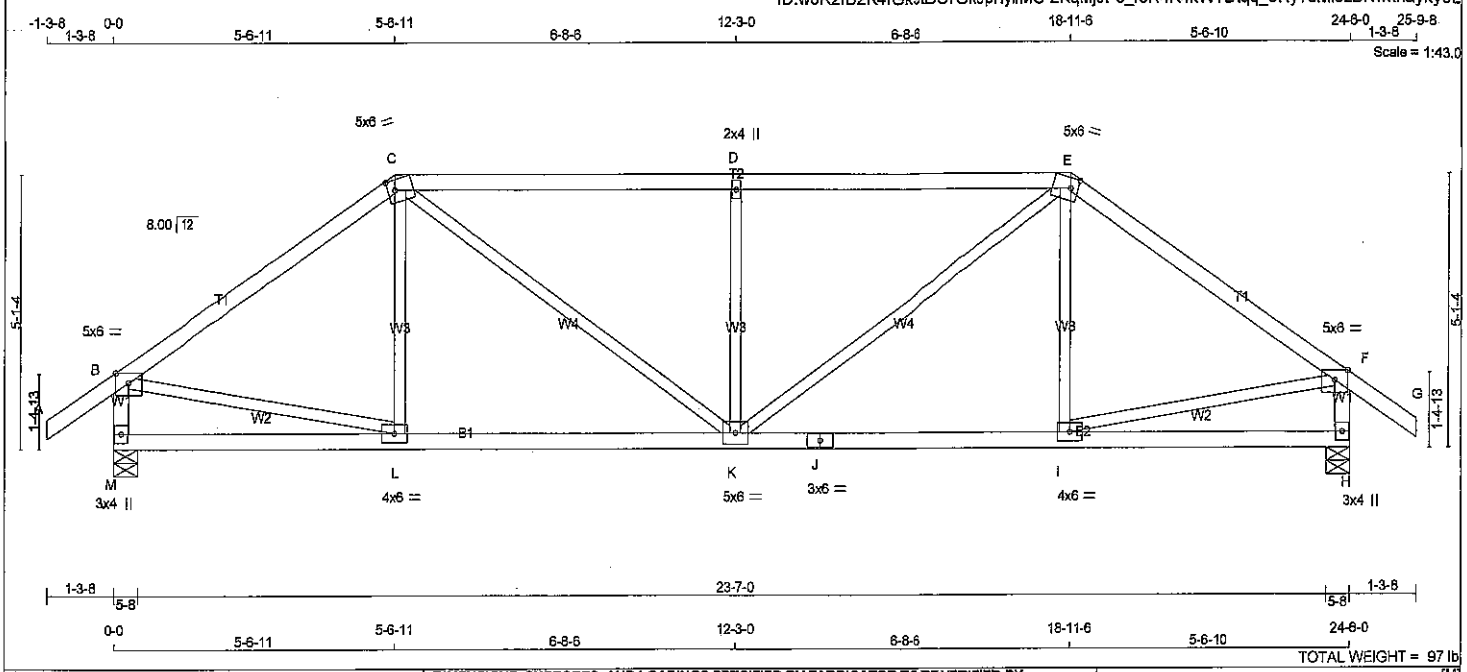

TOTAL WEIGHT = 97 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TTWW-m	MT20	5.0	6.0	2.25	1.75
D	TMW+w	MT20	2.0	4.0		
E	TTWW-m	MT20	5.0	6.0	2.25	1.75
F	TMVW-p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	5.0	6.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
M	1491	0	1491	0	0	5-8	5-8
H	1491	0	1491	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1208	697 / 0	257 / 0	0 / 0	0 / 0	253 / 0	0 / 0
H	1208	697 / 0	257 / 0	0 / 0	0 / 0	253 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.85 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD FROM	TO	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	L- C	-47 / 210	0.05 (3)
B- C	-1511 / 0	-84.3	-84.3	0.55 (1)	4.67	C- K	0 / 747	0.17 (1)
C- D	-1862 / 0	-84.3	-84.3	0.80 (1)	3.85	K- D	-692 / 0	0.27 (1)
D- E	-1862 / 0	-84.3	-84.3	0.80 (1)	3.85	K- E	0 / 747	0.17 (1)
E- F	-1511 / 0	-84.3	-84.3	0.55 (1)	4.67	I- E	-47 / 210	0.05 (3)
F- G	0 / 32	-84.3	-84.3	0.11 (1)	10.00	B- L	0 / 1278	0.29 (1)
M- B	-1430 / 0	0.0	0.0	0.15 (1)	6.83	I- F	0 / 1278	0.29 (1)
H- F	-1430 / 0	0.0	0.0	0.15 (1)	6.83			
M- L	0 / 0	-28.0	-28.0	0.25 (3)	10.00			
L- K	0 / 1255	-28.0	-28.0	0.40 (2)	10.00			
K- J	0 / 1255	-28.0	-28.0	0.40 (2)	10.00			
J- I	0 / 1255	-28.0	-28.0	0.40 (2)	10.00			
I- H	0 / 0	-28.0	-28.0	0.25 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.82")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.82")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.80 (C-D:1) , BC=0.40 (K-L:2) , WB=0.29 (B-L:1) , SSI=0.27 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (L) (INPUT = 0.90)
JSI METAL= 0.37 (J) (INPUT = 1.00)

DWG NO. TAM 41926 17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285473	T4	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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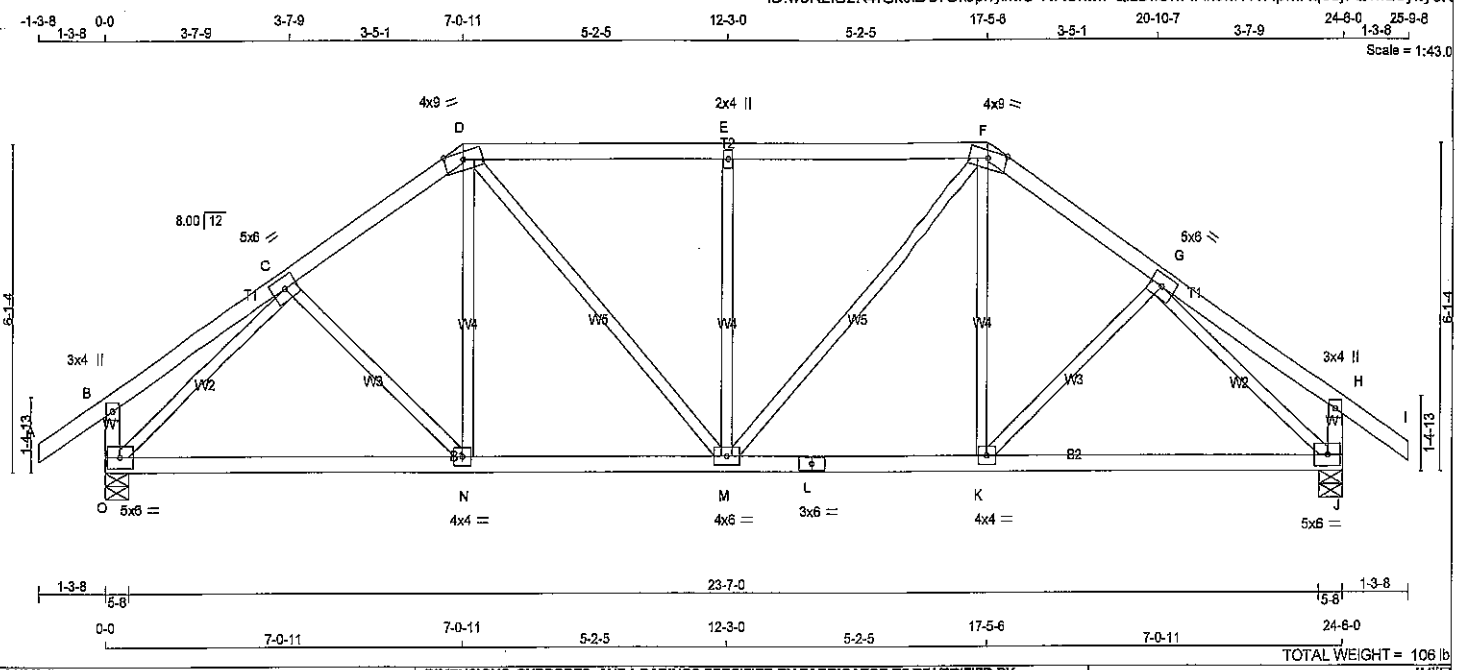

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW-m	MT20	4.0	9.0	Edge	
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	4.0	9.0	Edge	
G	TMWW-t	MT20	5.0	6.0		
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	5.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMVW1-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
O	1491	0	1491	0	0	5-8	5-8
J	1491	0	1491	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1208	697 / 0	257 / 0	0 / 0	0 / 0	253 / 0	0 / 0
J	1208	697 / 0	257 / 0	0 / 0	0 / 0	253 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.02 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	CSI (LC)	MAX UNBRAC LENGTH
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00
B-C	0 / 17	-84.3	-84.3	0.15 (1)	10.00
C-D	-1476 / 0	-84.3	-84.3	0.14 (1)	5.27
D-E	-1498 / 0	-84.3	-84.3	0.32 (1)	5.02
E-F	-1498 / 0	-84.3	-84.3	0.32 (1)	5.02
F-G	-1476 / 0	-84.3	-84.3	0.14 (1)	5.27
G-H	0 / 17	-84.3	-84.3	0.15 (1)	10.00
H-I	0 / 32	-84.3	-84.3	0.11 (1)	10.00
O-B	-233 / 0	0.0	0.0	0.02 (1)	7.81
J-H	-233 / 0	0.0	0.0	0.02 (1)	7.81
O-N	0 / 1208	-28.0	-28.0	0.45 (2)	10.00
N-M	0 / 1216	-28.0	-28.0	0.47 (2)	10.00
M-L	0 / 1216	-28.0	-28.0	0.47 (2)	10.00
L-K	0 / 1216	-28.0	-28.0	0.47 (2)	10.00
K-J	0 / 1208	-28.0	-28.0	0.45 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-N	0 / 119	0.03 (3)
N-D	0 / 304	0.07 (2)
D-M	0 / 424	0.10 (1)
M-E	-533 / 0	0.31 (1)
M-F	0 / 424	0.10 (1)
K-F	0 / 304	0.07 (2)
K-G	0 / 119	0.03 (3)
O-C	-1686 / 0	0.65 (1)
G-J	-1686 / 0	0.65 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.82")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.82")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.32 (D-E:1) , BC=0.47 (M-N:2) , WB=0.65 (C-O:1) , SSI=0.21 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.88 (F) (INPUT = 0.90)
JSI METAL= 0.41 (C) (INPUT = 1.00)

DWG NO. TAM41929-17
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285994	T5A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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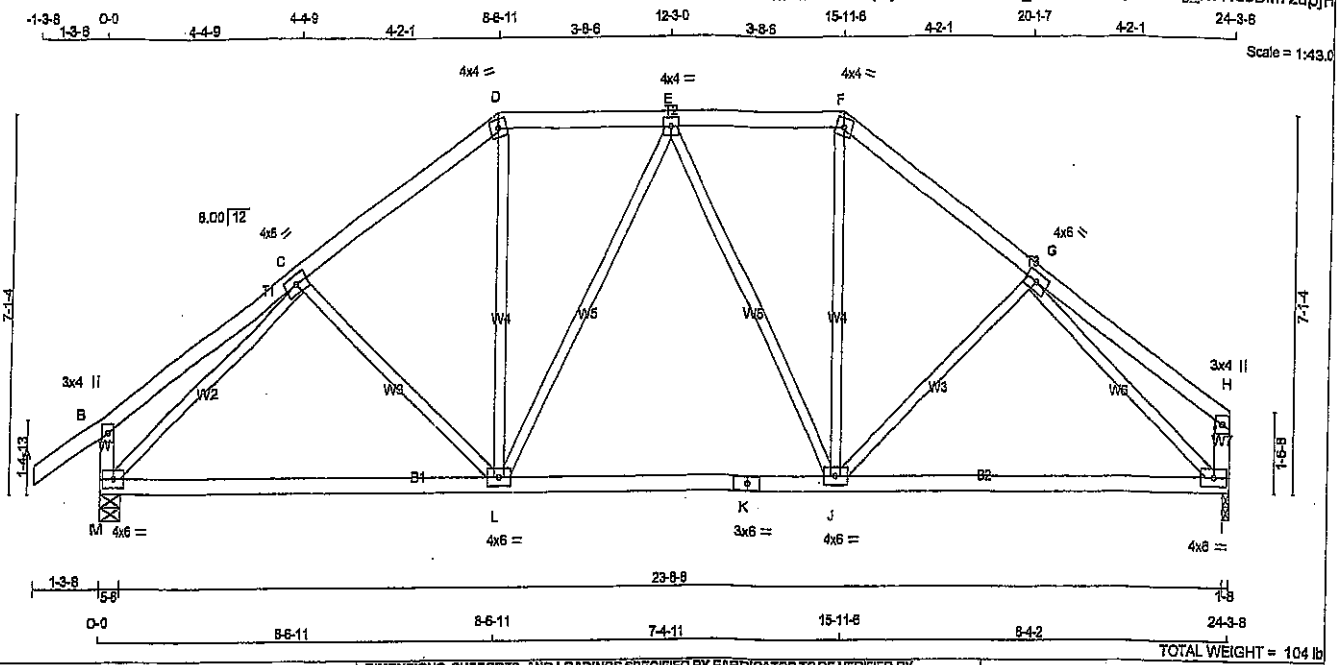

TOTAL WEIGHT = 104 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER		DESGR
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMV+p	MT20	3.0	4.0	
C	TMWW-t	MT20	4.0	6.0	
D	TTW-m	MT20	4.0	4.0	
E	TMWW-t	MT20	4.0	4.0	
F	TTW-m	MT20	4.0	4.0	
G	TMWW-t	MT20	4.0	6.0	
H	TMV+p	MT20	3.0	4.0	
I	BMVW1-t	MT20	4.0	6.0	
J	BMWWW-t	MT20	4.0	8.0	
K	BS-t	MT20	3.0	6.0	
L	BMWWW-t	MT20	4.0	6.0	
M	BMVW1-t	MT20	4.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
M	1479	0	1479	0	0	5-8	5-8
I	1363	0	1363	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1198	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0
I	1119	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.32 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	
B-C	0 / 22	-84.3	-84.3	0.24 (1)	10.00	
C-D	-1388 / 0	-84.3	-84.3	0.21 (1)	5.32	
D-E	-1142 / 0	-84.3	-84.3	0.15 (1)	5.79	
E-F	-1128 / 0	-84.3	-84.3	0.15 (1)	5.82	
F-G	-1371 / 0	-84.3	-84.3	0.20 (1)	5.35	
G-H	0 / 22	-84.3	-84.3	0.23 (1)	10.00	
M-B	-255 / 0	0.0	0.0	0.03 (1)	7.81	
I-H	-131 / 0	0.0	0.0	0.01 (1)	7.81	
M-L	0 / 1222	-28.0	-28.0	0.57 (2)	10.00	
L-K	0 / 1233	-28.0	-28.0	0.58 (2)	10.00	
K-J	0 / 1233	-28.0	-28.0	0.56 (2)	10.00	
J-I	0 / 1173	-28.0	-28.0	0.54 (2)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-L	-117 / 85	0.07 (1)
L-D	0 / 495	0.11 (1)
L-E	-195 / 0	0.24 (1)
E-J	-225 / 0	0.27 (1)
J-F	0 / 484	0.11 (1)
J-G	-70 / 102	0.04 (1)
M-C	-1660 / 0	0.93 (1)
G-I	-1632 / 0	0.88 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.8	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.23")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 761 (0.38")

CSI: TC=0.24/1.00 (B-C:1) , BC=0.58/1.00 (J-L:2) , WB=0.93/1.00 (C-M:1) , SSI=0.19/1.00 (L-M:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	522	2284	1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (M) (INPUT = 0.90)
JSI METAL= 0.40 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
11678
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 3985-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285994	T6A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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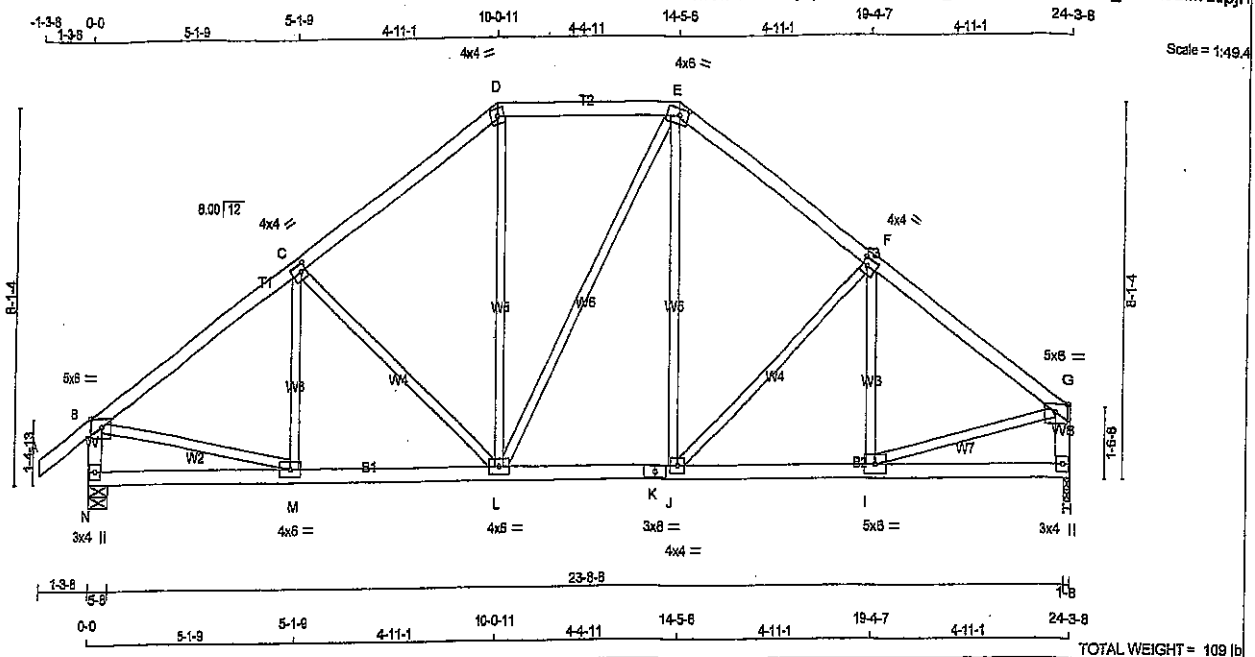

TOTAL WEIGHT = 109 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTW-m	MT20	4.0	4.0		
E	TTWW-m	MT20	4.0	6.0	1.75	2.50
F	TMWW-t	MT20	4.0	4.0	2.00	1.50
G	TMVW-p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	5.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	4.0	6.0		
M	BMWW-t	MT20	4.0	6.0		
N	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1479	0	1479	0	0	5-8	5-8
H	1363	0	1363	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1198	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0
H	1119	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00
B-C	-1531 / 0	-84.3	-84.3	0.31 (1)	5.00
C-D	-1271 / 0	-84.3	-84.3	0.29 (1)	5.39
D-E	-1037 / 0	-84.3	-84.3	0.22 (1)	5.91
E-F	-1282 / 0	-84.3	-84.3	0.28 (1)	5.42
F-G	-1483 / 0	-84.3	-84.3	0.29 (1)	5.08
N-B	-1420 / 0	0.0	0.0	0.15 (1)	6.84
H-G	-1307 / 0	0.0	0.0	0.14 (1)	7.08
N-M	0 / 0	-28.0	-28.0	0.18 (3)	10.00
M-L	0 / 1286	-28.0	-28.0	0.32 (2)	10.00
L-K	0 / 1030	-28.0	-28.0	0.23 (2)	10.00
K-J	0 / 1030	-28.0	-28.0	0.23 (2)	10.00
J-I	0 / 1257	-28.0	-28.0	0.31 (2)	10.00
I-H	0 / 0	-28.0	-28.0	0.17 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M-C	-115 / 147	0.04 (1)
C-L	-354 / 0	0.30 (1)
L-D	0 / 374	0.08 (2)
L-E	0 / 15	0.00 (1)
J-E	0 / 360	0.08 (2)
J-F	-310 / 0	0.26 (1)
I-F	-158 / 122	0.05 (1)
B-M	0 / 1325	0.30 (1)
I-G	0 / 1294	0.29 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.31/1.00 (B-C:1) , BC=0.32/1.00 (L-M:2) , WB=0.30/1.00 (C-L:1) , SSI=0.17/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (M) (INPUT = 0.90)
JSI METAL= 0.32 (M) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 3986-13
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285994	T7A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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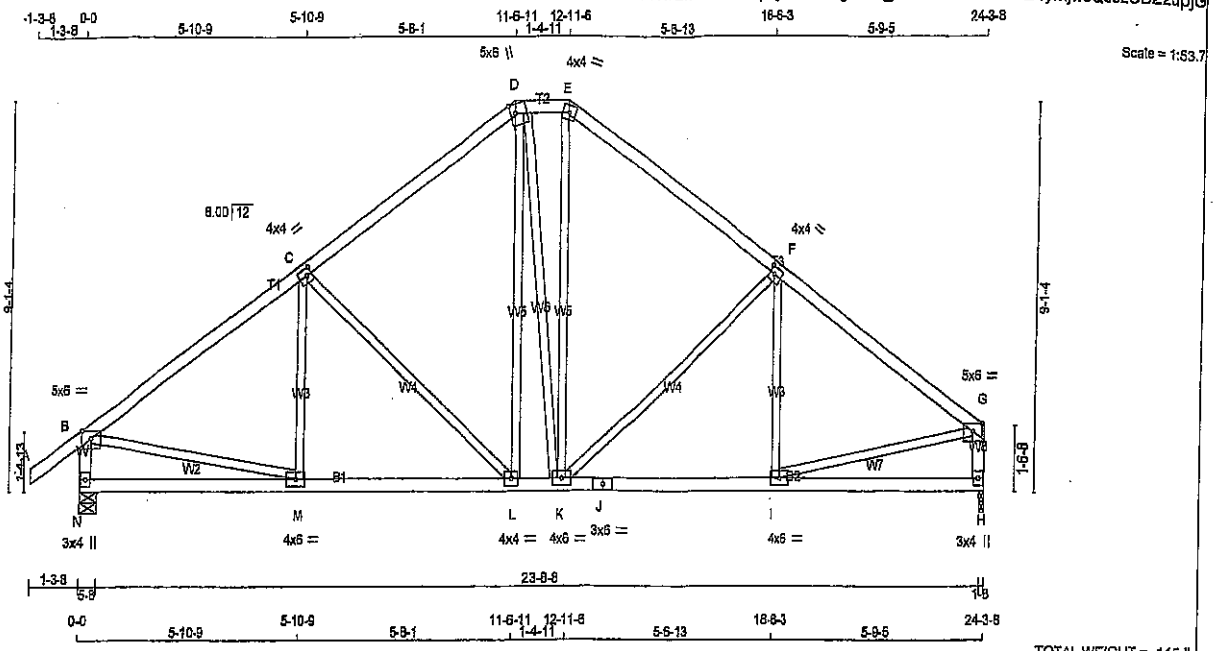

TOTAL WEIGHT = 115 lb [MiTek]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.50	1.50
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0	2.00	1.50
G	TMVW-p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMWW-t	MT20	4.0	6.0		
N	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1479	0	1479	0	0	5-8	5-8
H	1363	0	1363	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1198	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0
H	1119	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.86 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	
B-C	-1533 / 0	-84.3	-84.3	0.42 (1)	4.66	
C-D	-1163 / 0	-84.3	-84.3	0.38 (1)	5.44	
D-E	-945 / 0	-84.3	-84.3	0.03 (1)	6.25	
E-F	-1164 / 0	-84.3	-84.3	0.37 (1)	5.46	
F-G	-1491 / 0	-84.3	-84.3	0.39 (1)	4.94	
N-B	-1413 / 0	0.0	0.0	0.15 (1)	6.86	
H-G	-1299 / 0	0.0	0.0	0.14 (1)	7.08	
N-M	0 / 0	-28.0	-28.0	0.25 (3)	10.00	
M-L	0 / 1302	-28.0	-28.0	0.40 (2)	10.00	
L-K	0 / 943	-28.0	-28.0	0.26 (2)	10.00	
K-J	0 / 1268	-28.0	-28.0	0.37 (2)	10.00	
J-I	0 / 1268	-28.0	-28.0	0.37 (2)	10.00	
I-H	0 / 0	-28.0	-28.0	0.23 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M-C	-50 / 214	0.05 (3)
C-L	-481 / 0	0.57 (1)
L-D	0 / 377	0.08 (1)
D-K	0 / 17	0.00 (2)
K-E	0 / 390	0.09 (1)
K-F	-433 / 0	0.51 (1)
I-F	-94 / 185	0.04 (3)
B-M	0 / 1324	0.30 (1)
I-G	0 / 1296	0.29 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.42/1.00 (B-C:1) , BC=0.40/1.00 (L-M:2) , WB=0.57/1.00 (C-L:1) , SSI=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.83 (M) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
11678
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 3987-13
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285301	T8Z2	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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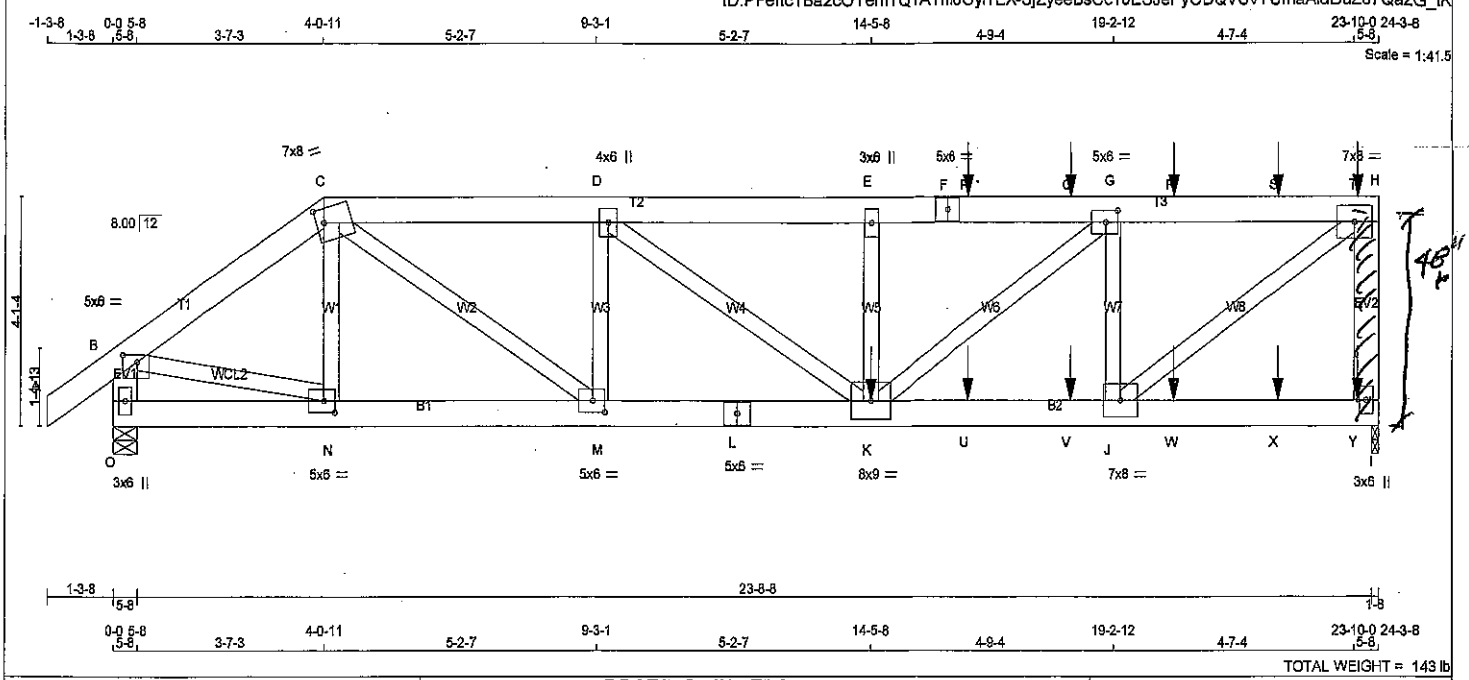

TOTAL WEIGHT = 143 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
I - H	2x6	DRY	No.2	SPF
O - B	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.25
C	TTWW-m	MT20	7.0	6.0	3.00	1.75
D	TMWW+t	MT20	4.0	6.0		
E	TMW+w	MT20	3.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMWW-t	MT20	5.0	6.0	2.50	2.75
H	TMVW-t	MT20	7.0	6.0		
I	BMV1+p	MT20	3.0	6.0		
J	BMWW-t	MT20	7.0	8.0		
K	BMWWW-t	MT20	8.0	9.0		
L	BS-t	MT20	5.0	6.0		
M	BMWW-t	MT20	5.0	6.0	2.50	2.75
N	BMWW-t	MT20	5.0	6.0	2.50	2.50
O	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 101.4 lbs FACTORED DOWN AT 16-4-12, 101.4 lbs FACTORED DOWN AT 18-4-12, 101.4 lbs FACTORED DOWN AT 20-4-12, AND 101.4 lbs FACTORED DOWN AT 22-4-12, AND 126.9 lbs FACTORED DOWN AT 23-10-12 ON TOP CHORD, AND 1454.0 lbs FACTORED DOWN AT 14-5-8, 69.9 lbs FACTORED DOWN AT 16-4-12, 69.9 lbs FACTORED DOWN AT 18-4-12, 69.9 lbs FACTORED DOWN AT 20-4-12, AND 69.9 lbs FACTORED DOWN AT 22-4-12, AND 84.7 lbs FACTORED DOWN AT 23-10-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

6) 48" VERTICAL SCAB

SCAB JOINT # I & O WITH 2 X 6 #2 SPF 24" LONG 1 SIDE(S) USING 2 ROW(S) 3 1/4" ARDOX SPIRAL/~~COMMON WIRE~~ NAILS AT 4" O/C STAGGERED (20 NAILS/SCAB). REQUIRED FOR 1 1/2" BRG PROVISION.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	2853	0	2853	0	0	1-8	1-8
O	2188	0	2188	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	2340	1303 / 0	531 / 0	0 / 0	0 / 0	507 / 0	0 / 0
O	1773	1023 / 0	378 / 0	0 / 0	0 / 0	372 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.55 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 33	-84.3	-84.3	0.07 (1)	10.00	N-C	-295 / 33	0.05 (1)
B-C	-2521 / 0	-84.3	-84.3	0.17 (1)	5.04	C-M	0 / 2343	0.41 (1)
C-D	-4011 / 0	-84.3	-84.3	0.27 (1)	4.08	M-D	-1174 / 0	0.20 (1)
D-E	-4986 / 0	-84.3	-84.3	0.31 (1)	3.67	D-K	0 / 1195	0.21 (1)
E-F	-4986 / 0	-84.3	-84.3	0.41 (1)	3.55	K-E	-417 / 0	0.07 (1)
F-P	-4986 / 0	-84.3	-84.3	0.41 (1)	3.55	K-G	0 / 2136	0.36 (1)
P-Q	-4986 / 0	-84.3	-84.3	0.41 (1)	3.55	J-G	-2079 / 0	0.36 (1)
Q-G	-4986 / 0	-84.3	-84.3	0.41 (1)	3.55	J-H	0 / 4052	0.72 (1)
G-R	-3290 / 0	-84.3	-84.3	0.33 (1)	4.35	B-N	0 / 2139	0.38 (1)
R-S	-3290 / 0	-84.3	-84.3	0.33 (1)	4.35			
S-T	-3290 / 0	-84.3	-84.3	0.33 (1)	4.35			
T-H	-3290 / 0	-84.3	-84.3	0.33 (1)	4.35			
I-H	-2720 / 0	0.0	0.0	0.40 (1)	6.29			
O-B	-2143 / 0	0.0	0.0	0.15 (1)	6.91			
O-N	0 / 0	-28.0	-28.0	0.08 (3)	10.00			
N-M	0 / 2080	-28.0	-28.0	0.31 (1)	10.00			
M-L	0 / 4011	-28.0	-28.0	0.61 (1)	10.00			
L-K	0 / 4011	-28.0	-28.0	0.61 (1)	10.00			
K-U	0 / 3290	-28.0	-28.0	0.53 (1)	10.00			
U-V	0 / 3290	-28.0	-28.0	0.53 (1)	10.00			
V-J	0 / 3290	-28.0	-28.0	0.53 (1)	10.00			
J-W	0 / 0	-28.0	-28.0	0.15 (3)	10.00			
W-X	0 / 0	-28.0	-28.0	0.15 (3)	10.00			
X-Y	0 / 0	-28.0	-28.0	0.15 (3)	10.00			
Y-I	0 / 0	-28.0	-28.0	0.15 (3)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	
K	14-5-8	-1454	-1454	---	BACK	VERT	TOTAL
P	16-4-12	-101	-101	---	BACK	VERT	TOTAL
Q	18-4-12	-101	-101	---	BACK	VERT	TOTAL
R	20-4-12	-101	-101	---	BACK	VERT	TOTAL
S	22-4-12	-101	-101	---	BACK	VERT	TOTAL
T	23-10-12	-127	-127	---	BACK	VERT	TOTAL
U	16-4-12	-40	-70	---	BACK	VERT	TOTAL
V	18-4-12	-40	-70	---	BACK	VERT	TOTAL
W	20-4-12	-40	-70	---	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER S. KATSOULAKOS PROVINCE OF ONTARIO 5/16

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.25")

CSI: TC=0.41/1.00 (E-G:1) , BC=0.61/1.00 (K-M:1) , WB=0.72/1.00 (H-J:1) , SSI=0.27/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	622	2264	1856

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.89 (L) (INPUT = 1.00)

Po 1/2

DWG NO. TAM 26864-18
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285301	T8Z2	1	1	TRUSS DESC.	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
X	22-4-12	-40	-70	—	BACK	VERT	TOTAL
Y	23-10-12	-48	-85	—	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM26964-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43994	DRWG NO.
285994	T9	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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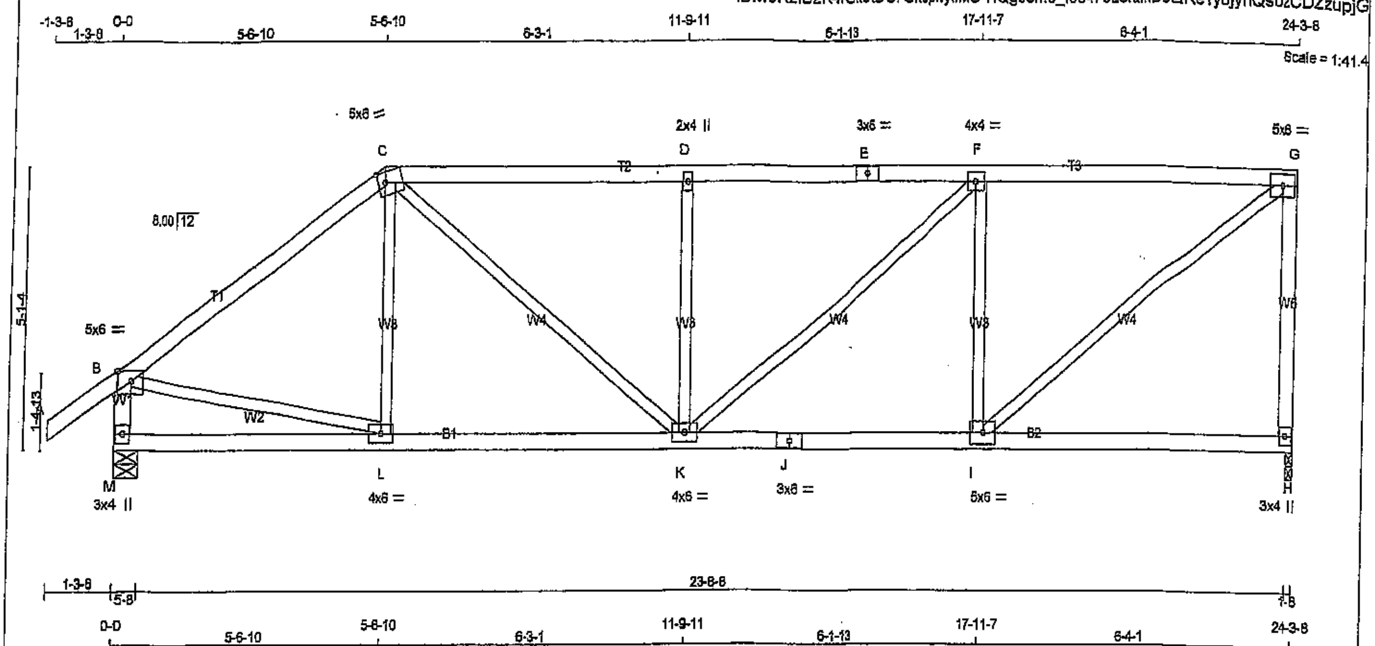

TOTAL WEIGHT = 2 X 99 = 197 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESGR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TTWW-m	MT20	5.0	6.0	2.25	1.75
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	1363	0	1363	0	0	1-8	1-8
M	1479	0	1479	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1119	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
M	1198	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.34 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00
B- C	-1493 / 0	-84.3	-84.3	0.55 (1)	4.69
C- D	-1797 / 0	-84.3	-84.3	0.55 (1)	4.37
D- E	-1797 / 0	-84.3	-84.3	0.56 (1)	4.34
E- F	-1797 / 0	-84.3	-84.3	0.56 (1)	4.34
F- G	-1416 / 0	-84.3	-84.3	0.53 (1)	4.81
H- G	-1282 / 0	0.0	0.0	0.57 (1)	7.10
M- B	-1417 / 0	0.0	0.0	0.15 (1)	6.85
M- L	0 / 0	-28.0	-28.0	0.24 (3)	10.00
L- K	0 / 1240	-28.0	-28.0	0.39 (2)	10.00
K- J	0 / 1416	-28.0	-28.0	0.44 (2)	10.00
J- I	0 / 1416	-28.0	-28.0	0.44 (2)	10.00
I- H	0 / 0	-28.0	-28.0	0.27 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- C	-49 / 204	0.05 (3)
B- L	0 / 1263	0.28 (1)
I- G	0 / 1778	0.40 (1)
C- K	0 / 702	0.16 (1)
I- F	-677 / 0	0.34 (1)
K- D	-566 / 0	0.22 (1)
K- F	0 / 484	0.11 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.57/1.00 (G-H:1) , BC=0.44/1.00 (I-K:2) , WB=0.40/1.00 (G-I:1) , SSI=0.25/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (K) (INPUT = 0.90)
JSI METAL= 0.44 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 3980 -18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285994	T10	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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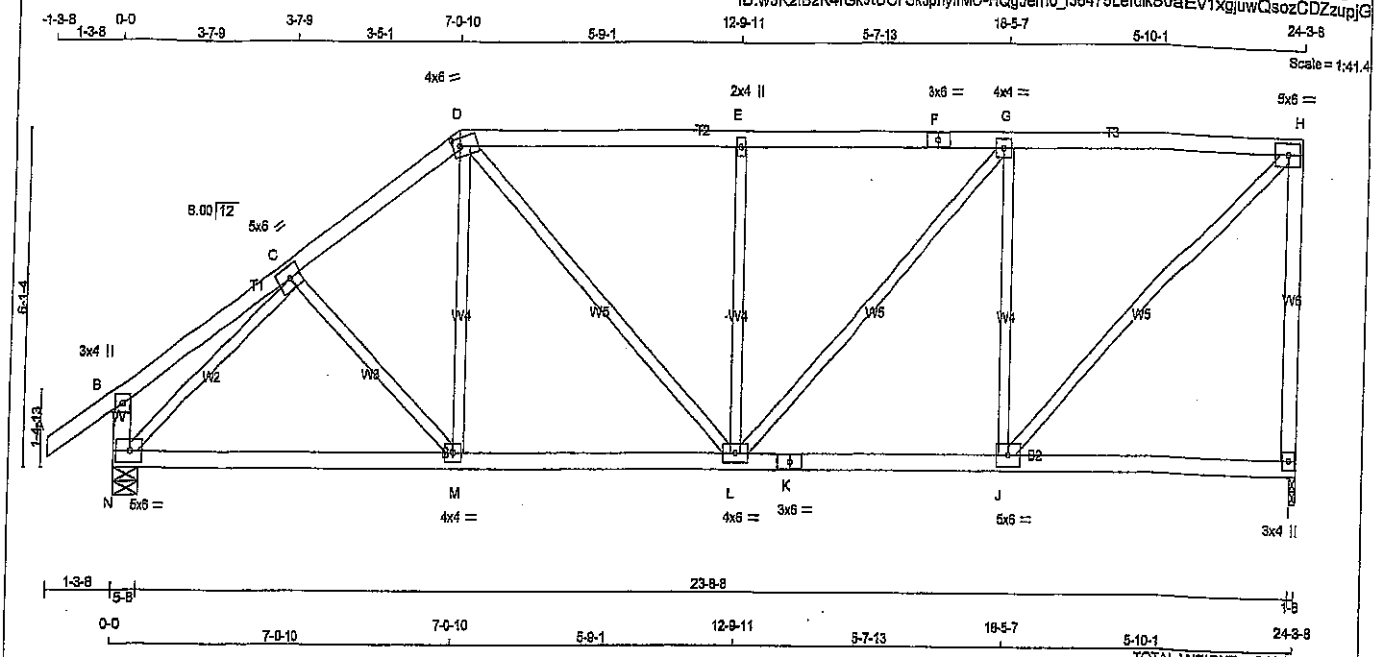


TOTAL WEIGHT = 2 X 106 = 213 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW-m	MT20	4.0	6.0	1.75	1.75
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TMVW-t	MT20	5.0	6.0		
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	5.0	6.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	4.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
I	1363	0	1363	0	0	1-8	1-8
N	1479	0	1479	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1119	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
N	1198	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.86 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.3	-84.3	0.11 (1)		10.00
B-C	0 / 17	-84.3	-84.3	0.15 (1)		10.00
C-D	-1460 / 0	-84.3	-84.3	0.18 (1)		5.22
D-E	-1473 / 0	-84.3	-84.3	0.43 (1)		4.86
E-F	-1473 / 0	-84.3	-84.3	0.44 (1)		4.86
F-G	-1473 / 0	-84.3	-84.3	0.44 (1)		4.86
G-H	-1103 / 0	-84.3	-84.3	0.42 (1)		5.46
I-H	-1298 / 0	0.0	0.0	0.94 (1)		7.08
N-B	-233 / 0	0.0	0.0	0.02 (1)		7.81
N-M	0 / 1196	-28.0	-28.0	0.45 (2)		10.00
M-L	0 / 1202	-28.0	-28.0	0.47 (2)		10.00
L-K	0 / 1103	-28.0	-28.0	0.37 (2)		10.00
K-J	0 / 1103	-28.0	-28.0	0.37 (2)		10.00
J-I	0 / 0	-28.0	-28.0	0.24 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-M	0 / 116	0.03 (3)
M-D	0 / 307	0.07 (2)
N-C	-1669 / 0	0.84 (1)
J-H	0 / 1557	0.35 (1)
D-L	0 / 385	0.09 (1)
J-G	-912 / 0	0.53 (1)
L-E	-520 / 0	0.30 (1)
L-G	0 / 530	0.12 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.94/1.00 (H-I:1) , BC=0.47/1.00 (L-M:2) , WB=0.64/1.00 (C-N:1) , SSI=0.23/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

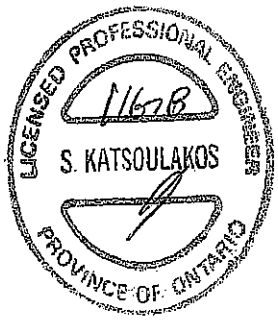
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (D) (INPUT = 0.90)
JSI METAL= 0.41 (C) (INPUT = 1.00)



DWG NO. TAM 3989 -18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285994	T11	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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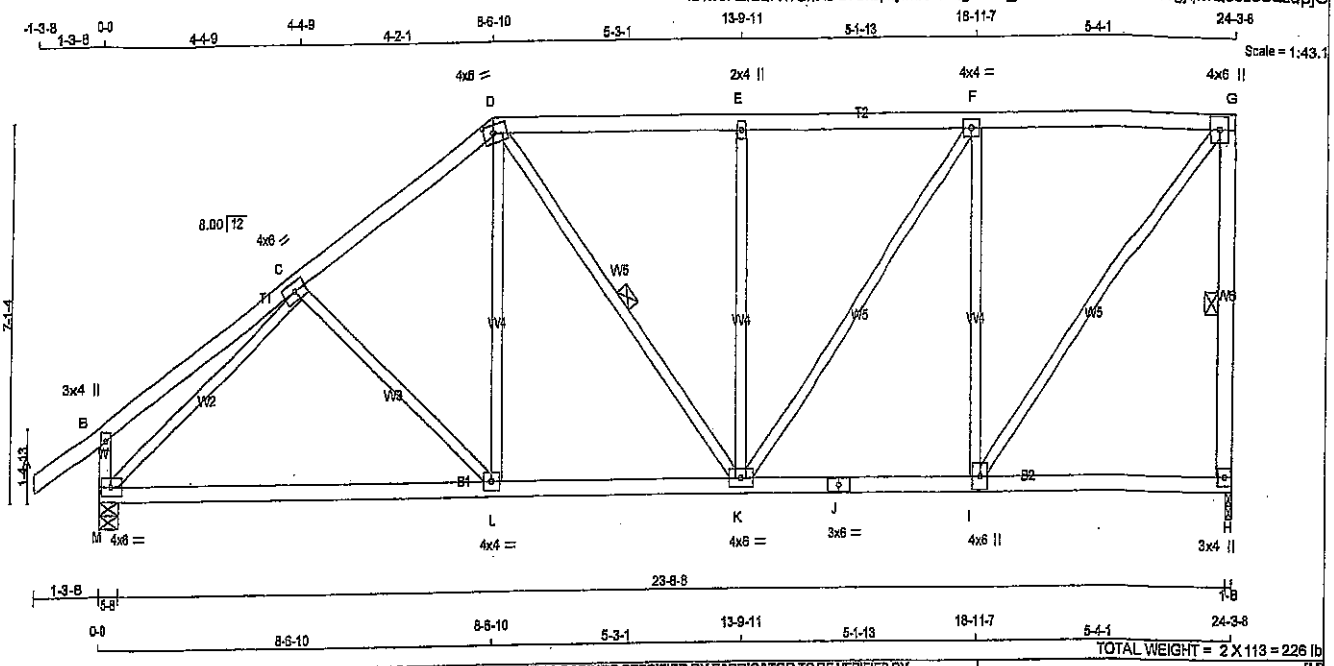


TOTAL WEIGHT = 2 X 113 = 226 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW-m	MT20	4.0	6.0	1.75	2.50
E	TMV+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW+p	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWW+t	MT20	4.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	1353	0	1353	0	0	1-8	1-8
M	1479	0	1479	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1119	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
M	1198	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

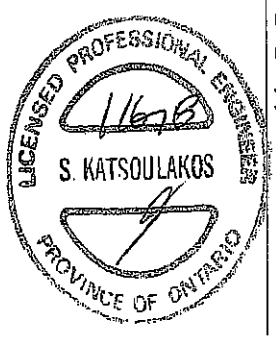
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-H, D-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	C-L	-117 / 85	0.07 (1)
B-C	0 / 23	-84.3	-84.3	0.24 (1)	10.00	L-D	0 / 414	0.09 (2)
C-D	-1390 / 0	-84.3	-84.3	0.28 (1)	5.20	M-C	-1681 / 0	0.93 (1)
D-E	-1228 / 0	-84.3	-84.3	0.34 (1)	5.37	I-G	0 / 1425	0.32 (1)
E-F	-1227 / 0	-84.3	-84.3	0.35 (1)	5.35	D-K	-5 / 141	0.03 (1)
F-G	-879 / 0	-84.3	-84.3	0.34 (1)	6.07	I-F	-946 / 0	0.83 (1)
H-G	-1305 / 0	0.0	0.0	0.30 (1)	5.66	K-E	-474 / 0	0.41 (1)
M-B	-255 / 0	0.0	0.0	0.03 (1)	7.81	K-F	0 / 576	0.13 (1)
M-L	0 / 1223	-28.0	-28.0	0.59 (2)	10.00			
L-K	0 / 1141	-28.0	-28.0	0.59 (2)	10.00			
K-J	0 / 879	-28.0	-28.0	0.31 (2)	10.00			
J-I	0 / 879	-28.0	-28.0	0.31 (2)	10.00			
I-H	0 / 0	-28.0	-28.0	0.21 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.23")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 779 (0.37")

CSI: TC=0.35/1.00 (E-F:1) , BC=0.59/1.00 (K-L:2) , WB=0.93/1.00 (C-M:1) , SSI=0.21/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (M) (INPUT = 0.90)
JSI METAL= 0.40 (C) (INPUT = 1.00)

DWG NO. TAM 3990 -18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285994	T12	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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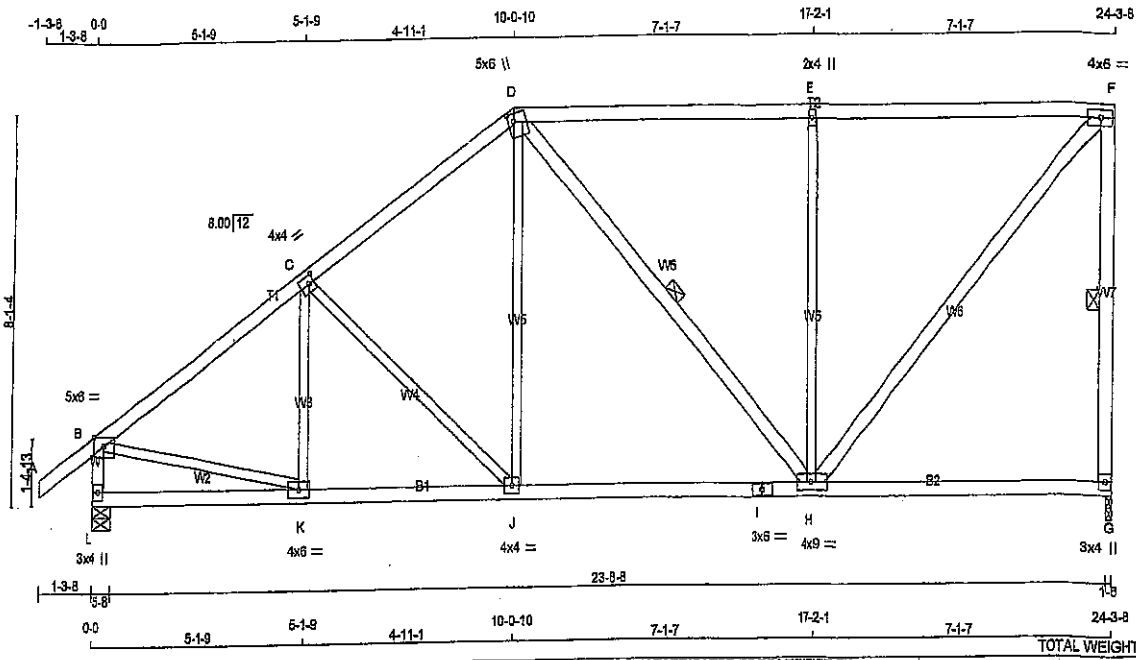


TOTAL WEIGHT = 2 X 117 = 235 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMV+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	8.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1363	0	1363	0	0	1-8	1-8
L	1479	0	1479	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1119	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
L	1198	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.81 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

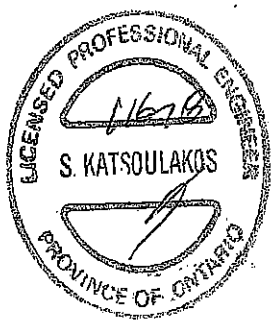
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO								
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	K-C	-123 / 133	0.04 (1)
B-C	-1529 / 0	-84.3	-84.3	0.43 (1)	4.83	C-J	-347 / 0	0.29 (1)
C-D	-1275 / 0	-84.3	-84.3	0.40 (1)	5.20	J-D	0 / 436	0.10 (2)
D-E	-967 / 0	-84.3	-84.3	0.80 (1)	4.81	D-H	-110 / 0	0.06 (1)
E-F	-967 / 0	-84.3	-84.3	0.81 (1)	4.81	H-E	-745 / 0	0.94 (1)
G-F	-1284 / 0	0.0	0.0	0.38 (1)	5.69	H-F	0 / 1432	0.23 (1)
L-B	-1418 / 0	0.0	0.0	0.15 (1)	6.85	B-K	0 / 1323	0.30 (1)
L-K	0 / 0	-28.0	-28.0	0.16 (3)	10.00			
K-J	0 / 1295	-28.0	-28.0	0.33 (2)	10.00			
J-I	0 / 1041	-28.0	-28.0	0.49 (2)	10.00			
I-H	0 / 1041	-28.0	-28.0	0.49 (2)	10.00			
H-G	0 / 0	-28.0	-28.0	0.37 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.81/1.00 (E-F:1) , BC=0.49/1.00 (H-J:2) , WB=0.94/1.00 (E-H:1) , SSI=0.29/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	818	354	1687	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL= 0.36 (I) (INPUT = 1.00)

DWG NO. TAM 3991-13
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
286039	T13	5	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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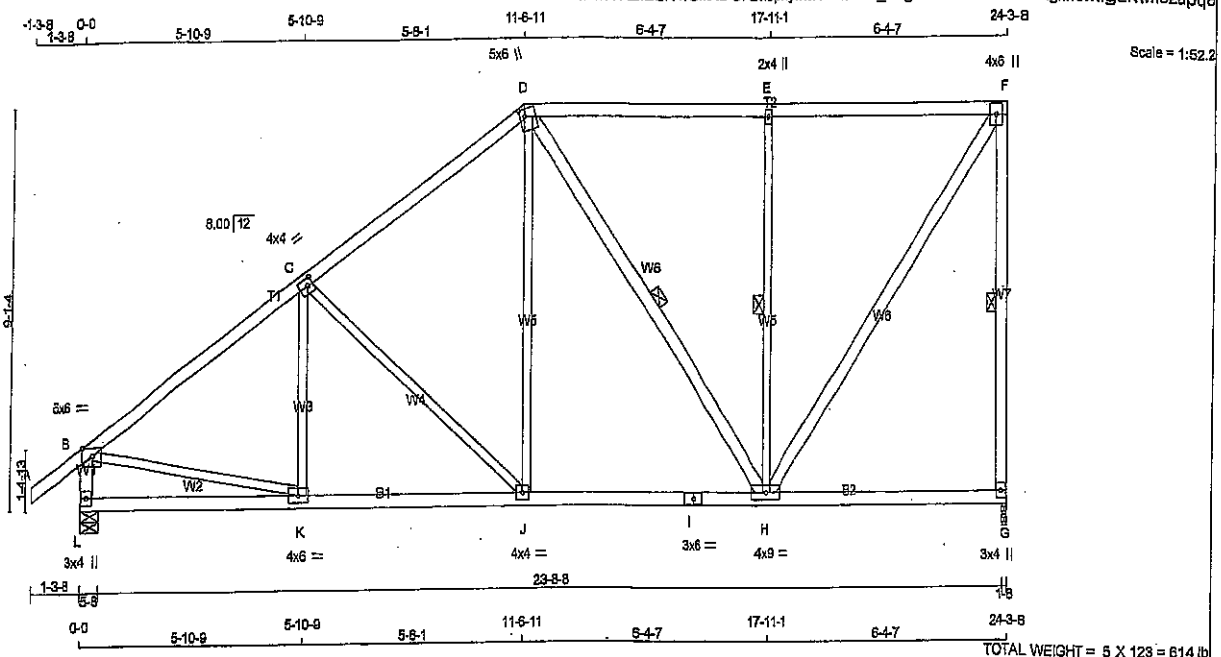


TOTAL WEIGHT = 5 X 123 = 614 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	8.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1363	0	1363	0	0	1-8	1-8
L	1479	0	1479	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1119	621 / 0	255 / 0	0 / 0	0 / 0	243 / 0	0 / 0
L	1168	692 / 0	255 / 0	0 / 0	0 / 0	251 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.61 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

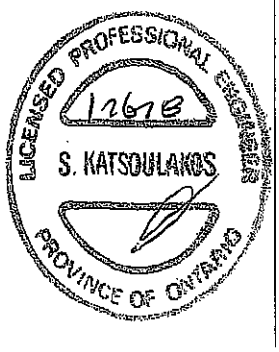
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	K-C	-56 / 203	0.05 (3)
B-C	-1531 / 0	-84.3	-84.3	0.59 (1)	4.61	C-J	-474 / 0	0.56 (1)
C-D	-1167 / 0	-84.3	-84.3	0.54 (1)	5.16	J-D	0 / 490	0.11 (2)
D-E	-790 / 0	-84.3	-84.3	0.61 (1)	5.73	D-H	-270 / 0	0.17 (1)
E-F	-790 / 0	-84.3	-84.3	0.62 (1)	5.73	H-E	-666 / 0	0.36 (1)
G-F	-1292 / 0	0.0	0.0	0.50 (1)	5.66	H-F	0 / 1344	0.22 (1)
L-B	-1412 / 0	0.0	0.0	0.15 (1)	6.86	B-K	0 / 1322	0.30 (1)
L-K	0 / 0	-28.0	-28.0	0.23 (3)	10.00			
K-J	0 / 1300	-28.0	-28.0	0.38 (2)	10.00			
J-I	0 / 948	-28.0	-28.0	0.40 (2)	10.00			
I-H	0 / 946	-28.0	-28.0	0.40 (2)	10.00			
H-G	0 / 0	-28.0	-28.0	0.29 (3)	10.00			



DESIGN CRITERIA [M]

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.62/1.00 (E-F:1) , BC=0.40/1.00 (H-J:2) , WB=0.58/1.00 (C-J:1) , SSI=0.26/1.00 (E-F:1)

OOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.32 (K) (INPUT = 1.00)

DWG NO. TAM 3993 -13
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285479	T14	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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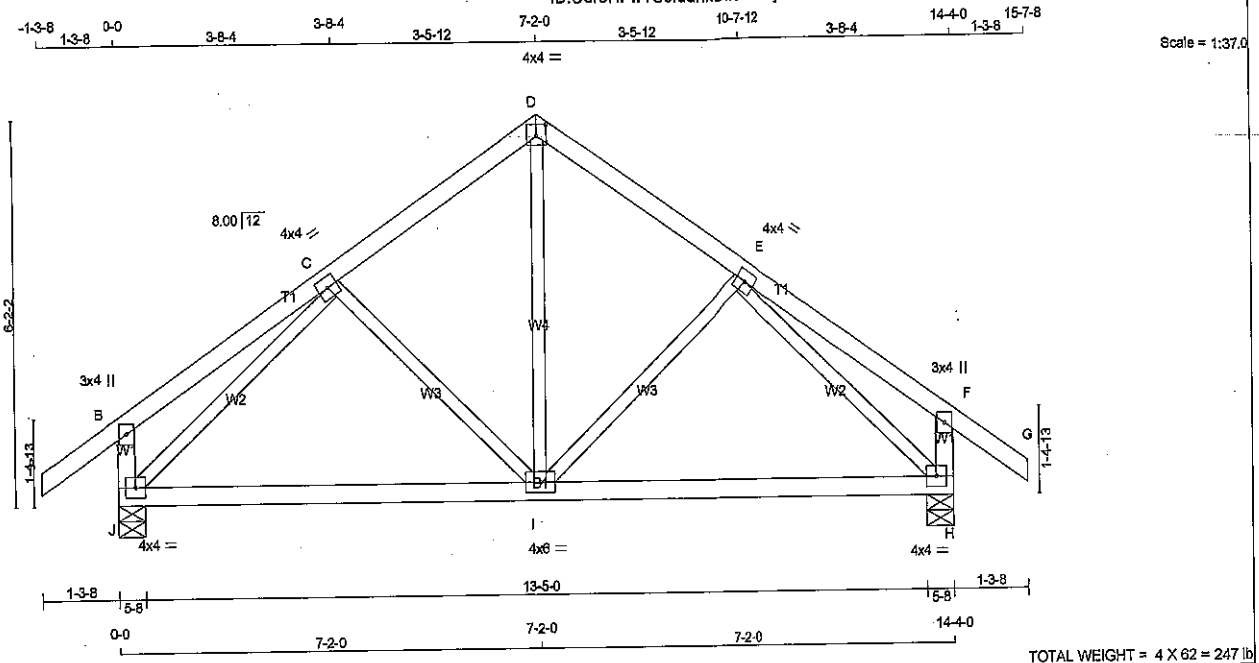

TOTAL WEIGHT = 4 X 62 = 247 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	920	0	920	0	0	5-8	5-8
H	920	0	920	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	739	437 / 0	151 / 0	0 / 0	0 / 0	152 / 0	0 / 0
H	739	437 / 0	151 / 0	0 / 0	0 / 0	152 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	CSI	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 32	-84.3	-84.3	1	0.11 (1)	10.00	I-D	0 / 460	0.10 (1)
B-C	0 / 19	-84.3	-84.3	1	0.17 (1)	10.00	I-E	-152 / 44	0.06 (1)
C-D	-645 / 0	-84.3	-84.3	1	0.13 (1)	6.25	C-I	-152 / 44	0.06 (1)
D-E	-645 / 0	-84.3	-84.3	1	0.13 (1)	6.25	J-C	-879 / 0	0.35 (1)
E-F	0 / 19	-84.3	-84.3	1	0.17 (1)	10.00	E-H	-879 / 0	0.35 (1)
F-G	0 / 32	-84.3	-84.3	1	0.11 (1)	10.00			
J-B	-232 / 0	0.0	0.0	1	0.02 (1)	7.81			
H-F	-232 / 0	0.0	0.0	1	0.02 (1)	7.81			
J-I	0 / 631	-28.0	-28.0	2	0.49 (2)	10.00			
I-H	0 / 631	-28.0	-28.0	2	0.49 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.48")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.48")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.17/1.00 (E-F:1) , BC=0.49/1.00 (H-I:2) , WB=0.35/1.00 (E-H:1) , SSI=0.17/1.00 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (D) (INPUT = 0.90)
JSI METAL= 0.31 (C) (INPUT = 1.00)

DWG NO. TAM 21165 -18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285473	T15	2	1	TRUSS DESC.	

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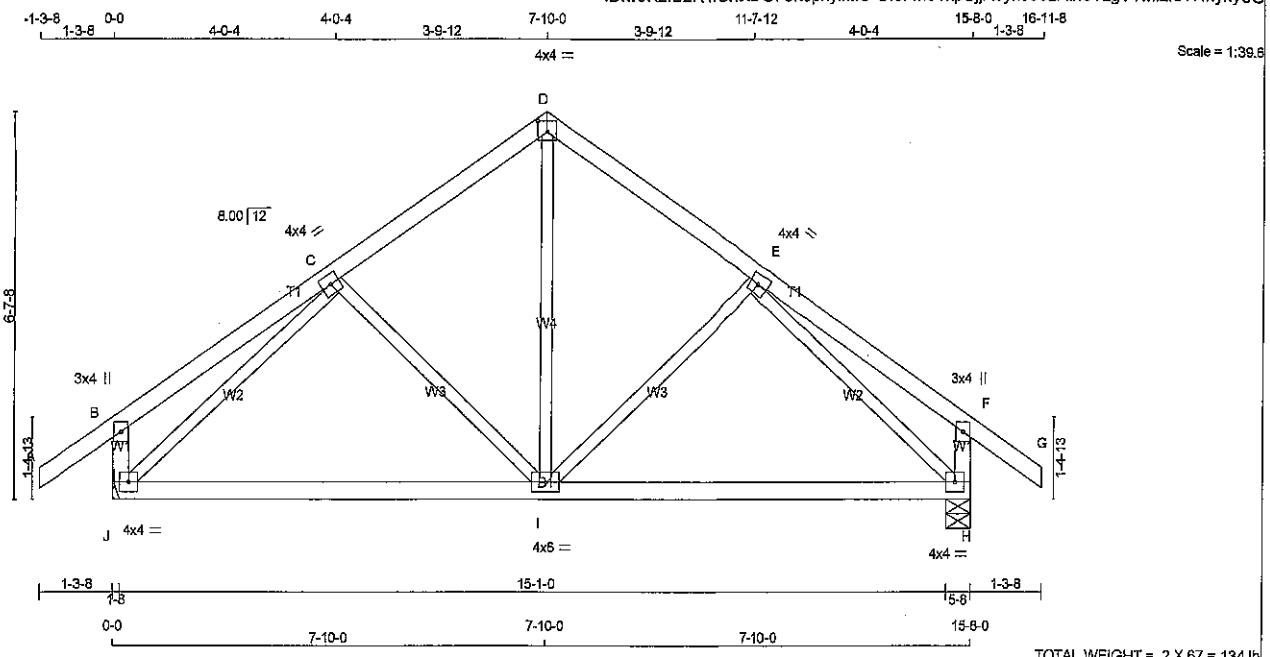

TOTAL WEIGHT = 2 X 67 = 134 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
J	995	0	995	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
H	995	0	995	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	800	471 / 0	165 / 0	0 / 0	0 / 0	165 / 0	0 / 0
H	800	471 / 0	165 / 0	0 / 0	0 / 0	165 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	
B- C	0 / 21	-84.3	-84.3	0.20 (1)	10.00	
C- D	-717 / 0	-84.3	-84.3	0.16 (1)	6.25	
D- E	-717 / 0	-84.3	-84.3	0.16 (1)	6.25	
E- F	0 / 21	-84.3	-84.3	0.20 (1)	10.00	
F- G	0 / 32	-84.3	-84.3	0.11 (1)	10.00	
J- B	-243 / 0	0.0	0.0	0.02 (1)	7.81	
H- F	-243 / 0	0.0	0.0	0.02 (1)	7.81	
J- I	0 / 709	-28.0	-28.0	0.58 (2)	10.00	
I- H	0 / 709	-28.0	-28.0	0.58 (2)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- D	0 / 516	0.12 (1)
I- E	-179 / 43	0.09 (1)
C- I	-179 / 43	0.09 (1)
J- C	-975 / 0	0.46 (1)
E- H	-975 / 0	0.46 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.52")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.52")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.20 (B-C:1) , BC=0.58 (H-I:2) , WB=0.46 (C-J:1) , SSI=0.19 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (D) (INPUT = 0.90)
JSI METAL= 0.35 (C) (INPUT = 1.00)

DWG NO. TAM41990-17
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285473	G15	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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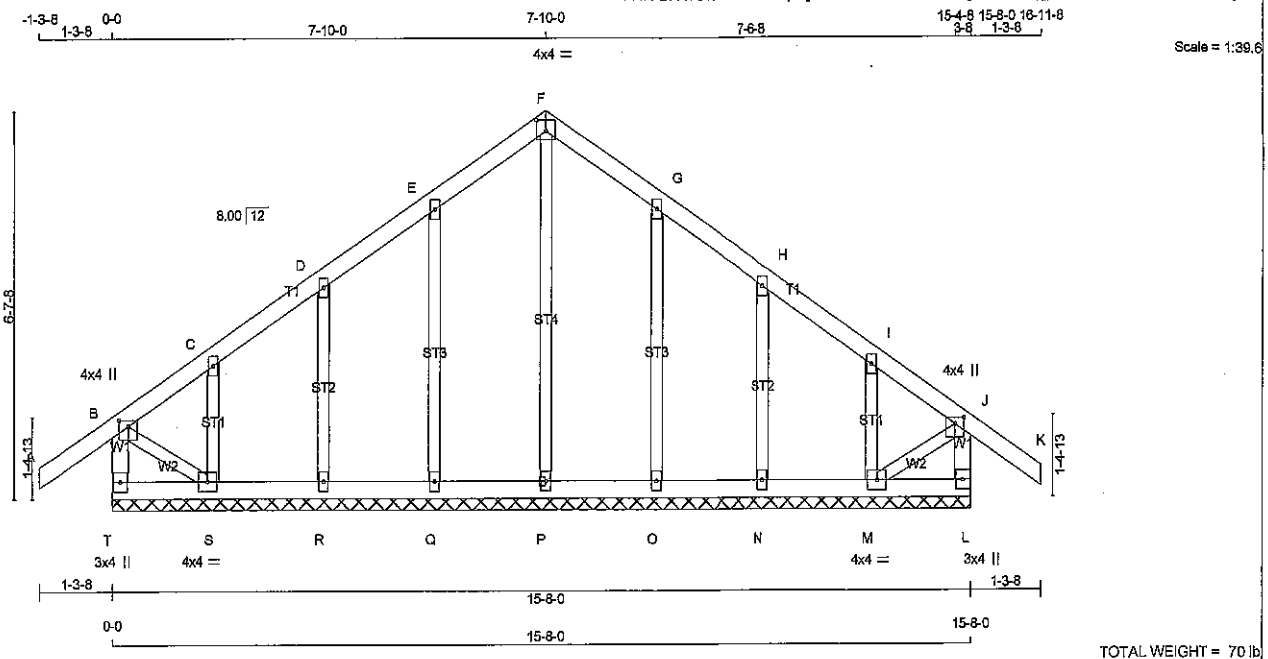

TOTAL WEIGHT = 70 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
T - B	2x4	DRY	No.2	SPF
A - F	2x4	DRY	No.2	SPF
F - K	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C, D, E, G, H, I						
C	TMW+w	MT20	2.0	4.0		
F	TTW-p	MT20	4.0	4.0	2.25	2.00
J	TMVW+p	MT20	4.0	4.0	1.25	2.00
L	BMV1+p	MT20	3.0	4.0		
M	BMWW1-t	MT20	4.0	4.0		
N, O, P, Q, R						
N	BMW1+w	MT20	2.0	4.0		
S	BMWW1-t	MT20	4.0	4.0		
T	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
T- B	-253 / 0	0.0	0.0	0.03 (1)	7.81
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00
B- C	-46 / 0	-84.3	-84.3	0.11 (1)	6.25
C- D	-15 / 0	-84.3	-84.3	0.04 (1)	6.25
D- E	-14 / 0	-84.3	-84.3	0.05 (1)	6.25
E- F	-21 / 0	-84.3	-84.3	0.05 (1)	6.25
F- G	-21 / 0	-84.3	-84.3	0.05 (1)	6.25
G- H	-14 / 0	-84.3	-84.3	0.05 (1)	6.25
H- I	-15 / 0	-84.3	-84.3	0.04 (1)	6.25
I- J	-46 / 0	-84.3	-84.3	0.11 (1)	6.25
J- K	0 / 32	-84.3	-84.3	0.11 (1)	10.00
L- J	-253 / 0	0.0	0.0	0.03 (1)	7.81
T- S	0 / 0	-28.0	-28.0	0.03 (2)	10.00
S- R	0 / 16	-28.0	-28.0	0.03 (2)	10.00
R- Q	0 / 12	-28.0	-28.0	0.02 (2)	10.00
Q- P	0 / 9	-28.0	-28.0	0.02 (2)	10.00
P- O	0 / 9	-28.0	-28.0	0.02 (2)	10.00
O- N	0 / 12	-28.0	-28.0	0.02 (2)	10.00
N- M	0 / 16	-28.0	-28.0	0.03 (2)	10.00
M- L	0 / 0	-28.0	-28.0	0.03 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- F	-129 / 0	0.09 (1)
Q- E	-189 / 0	0.08 (1)
R- D	-172 / 0	0.04 (1)
S- C	-108 / 0	0.02 (1)
Q- G	-189 / 0	0.08 (1)
N- H	-172 / 0	0.04 (1)
M- I	-108 / 0	0.02 (1)
B- S	0 / 23	0.01 (1)
M- J	0 / 23	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11 (J-K:1) , BC=0.03 (R-S:2) , WB=0.09 (F-P:1) , SSI=0.08 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (B) (INPUT = 0.90)
JSI METAL= 0.05 (G) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM41985-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285473	T90TCX	20	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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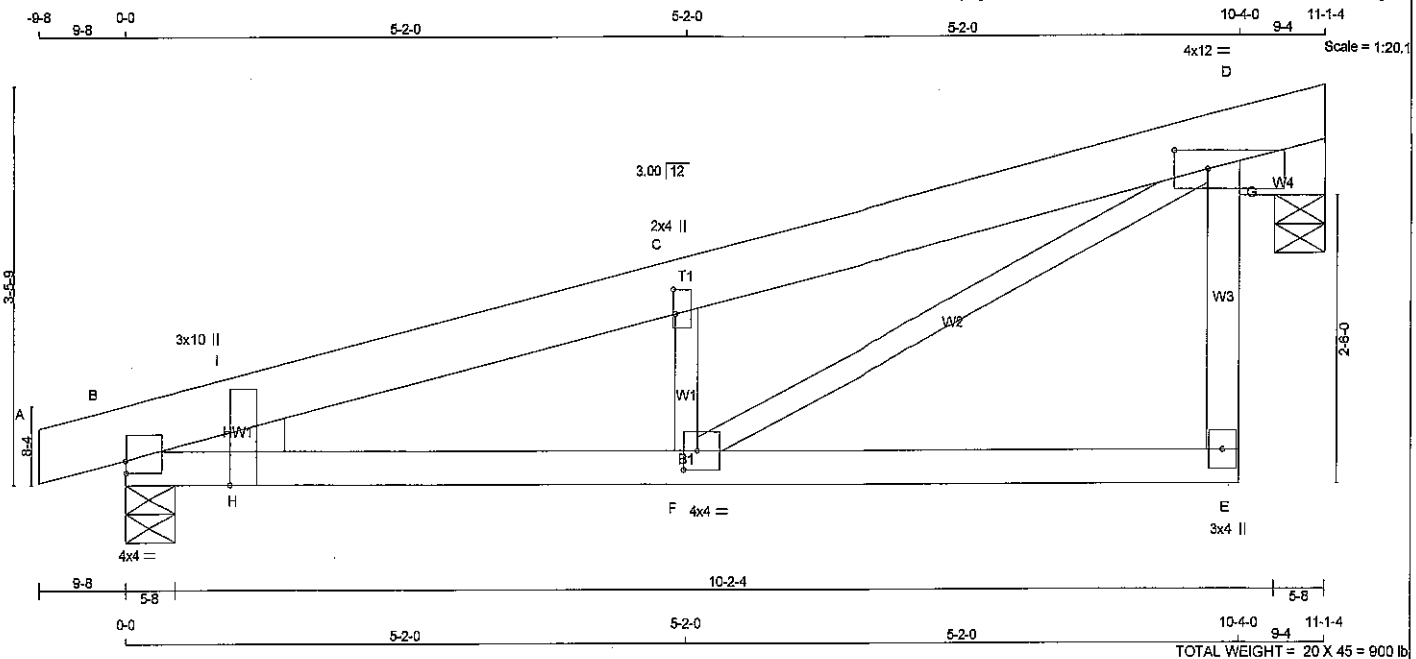

TOTAL WEIGHT = 20 X 45 = 900 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	4.0	4.0	1.25	
B	WP+I	MT20	3.0	10.0	Edge	11.50
C	TMW+w	MT20	2.0	4.0	2.50	0.25
D	TMVWW1-p	MT20	4.0	12.0	2.00	3.75
E	BMV+p	MT20	3.0	4.0		
F	BMWW-t	MT20	4.0	4.0	2.00	1.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG	HEEL
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	WEDGE
J(D)	551	0	551	0	0	5-8	5-8	
B	660	0	660	0	0	5-8	5-8	2x4 L

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J(D)	452	251 / 0	103 / 0	0 / 0	0 / 0	98 / 0	0 / 0
B	548	321 / 0	114 / 0	0 / 0	0 / 0	114 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J(D), B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-4 / 0	-84.3	-84.3	0.02 (1)	10.00	F- C	-420 / 0	0.06 (1)	
B- I	-1258 / 0	-84.3	-84.3	0.03 (1)	6.25	F- D	0 / 1157	0.26 (1)	
I- C	-1158 / 0	-84.3	-84.3	0.07 (1)	6.25	H- I	0 / 131	0.00 (1)	
C- D	-1152 / 0	-84.3	-84.3	0.17 (1)	8.25	D- J	-785 / 0	0.03 (1)	
E- G	0 / 106	0.0	0.0	0.34 (1)	10.00	G- J	0 / 558	0.00 (1)	
G- D	0 / 106	0.0	0.0	0.34 (1)	10.00				
B- H	0 / 1128	-28.0	-28.0	0.20 (1)	10.00				
H- F	0 / 1128	-28.0	-28.0	0.28 (2)	10.00				
F- E	0 / 103	-28.0	-28.0	0.17 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD,

ALLOWABLE DEFL.(LL)= L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.34 (E-G:1) , BC=0.28 (F-H:2) , WB=0.26 (D-F:1) , SSI=0.15 (D-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (F) (INPUT = 0.90)
JSI METAL= 0.40 (B) (INPUT = 1.00)

DWG NO. TAM-4194-17

STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285473	G90	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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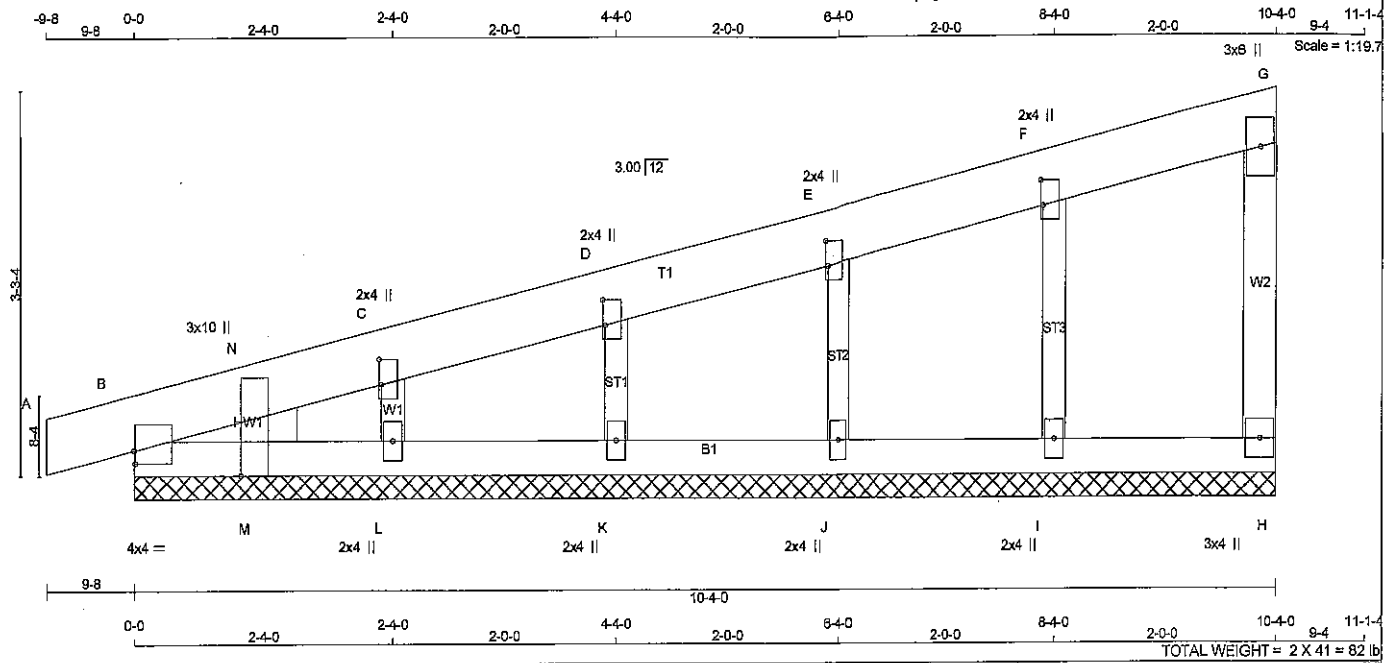

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - G	2x6	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
B - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	4.0	4.0	1.25	
B	WP+I	MT20	3.0	10.0	Edge	11.50
C, D, E, F						
C	TMW+w	MT20	2.0	4.0	2.50	0.25
G	TMV+p	MT20	3.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I, J, K, L						
I	BMW1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

HEEL
WEDGE
2x4 L
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF) CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-4 / 0	-84.3	-84.3	0.02 (1)	10.00	I- F	-186 / 0	0.03 (1)
B- N	-46 / 0	-84.3	-84.3	0.02 (1)	6.25	J- E	-188 / 0	0.03 (1)
N- C	-14 / 3	-84.3	-84.3	0.03 (1)	6.25	K- D	-158 / 0	0.02 (1)
C- D	-18 / 0	-84.3	-84.3	0.03 (1)	6.25	L- C	-201 / 0	0.03 (1)
D- E	-9 / 0	-84.3	-84.3	0.02 (1)	10.00	M- N	0 / 41	0.00 (1)
E- F	-4 / 0	-84.3	-84.3	0.02 (1)	10.00			
F- G	-4 / 0	-84.3	-84.3	0.02 (1)	10.00			
H- G	-72 / 0	0.0	0.0	0.01 (1)	7.81			
B- M	0 / 28	-28.0	-28.0	0.02 (1)	10.00			
M- L	0 / 28	-28.0	-28.0	0.03 (2)	10.00			
L- K	0 / 16	-28.0	-28.0	0.02 (2)	10.00			
K- J	0 / 9	-28.0	-28.0	0.02 (2)	10.00			
J- I	0 / 4	-28.0	-28.0	0.03 (2)	10.00			
I- H	0 / 0	-28.0	-28.0	0.03 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.03 (C-D:1) , BC=0.03 (I-J:2) , WB=0.03 (F-I:1) , SSI=0.07 (C-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (C) (INPUT = 0.90)
JSI METAL= 0.04 (L) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM-41986-17
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285473	T105	6	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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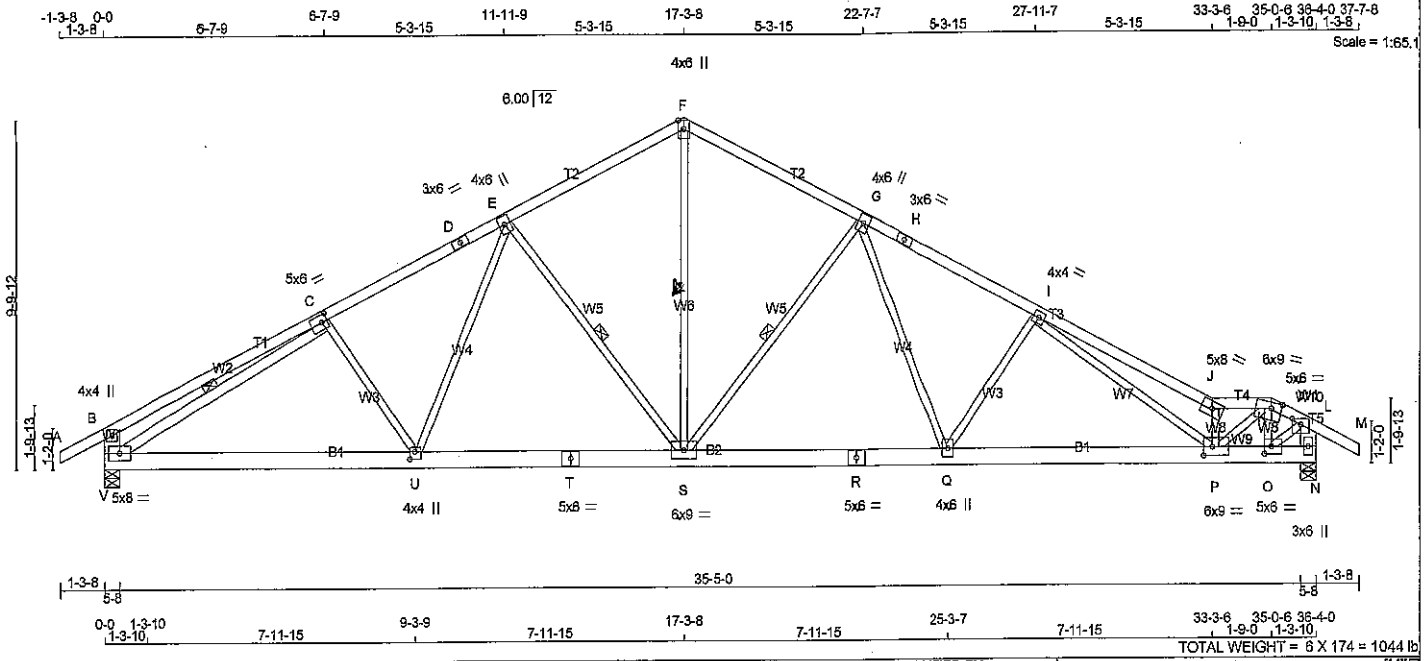


TOTAL WEIGHT = 6 X 174 = 1044 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
J - K	2x4	DRY	No.2	SPF
K - M	2x4	DRY	No.2	SPF
V - B	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
V - T	2x6	DRY	No.2	SPF
T - R	2x6	DRY	No.2	SPF
R - N	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
V - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.00
D	TS-t	MT20	3.0	6.0		
E	TMWW+t	MT20	4.0	6.0		
F	TTW+p	MT20	4.0	6.0	Edge	
G	TMWW+t	MT20	4.0	6.0		
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	4.0	4.0		
J	TTW-h	MT20	5.0	8.0		
K	TTWW-m	MT20	6.0	9.0	2.00	3.50
L	TMVW-p	MT20	5.0	6.0	2.00	3.00
N	BMV1+p	MT20	3.0	6.0		
O	BMWW-t	MT20	5.0	6.0	2.50	2.50
P	BMWWW-t	MT20	6.0	9.0	3.00	3.00
Q	BMWW+t	MT20	4.0	6.0		
R	BS-t	MT20	5.0	6.0		
S	BMWWW-t	MT20	6.0	9.0		
T	BS-t	MT20	5.0	6.0		
U	BMWW+t	MT20	4.0	4.0	2.50	1.75
V	BMVW1-t	MT20	5.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	2156	0	2156	0	0	5-8	5-8
V	2151	0	2151	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1753	1000 / 0	382 / 0	0 / 0	0 / 0	372 / 0	0 / 0
V	1751	998 / 0	382 / 0	0 / 0	0 / 0	371 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, V

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.06 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-S, G-S, C-V.

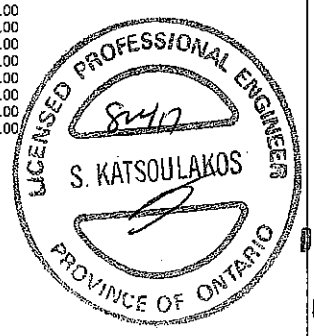
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 26	-84.3	-84.3	0.11 (1)		10.00
B- C	0 / 21	-84.3	-84.3	0.43 (1)		10.00
C- D	-2936 / 0	-84.3	-84.3	0.49 (1)		3.66
D- E	-2936 / 0	-84.3	-84.3	0.49 (1)		3.66
E- F	-2220 / 0	-84.3	-84.3	0.36 (1)		4.27
F- G	-2222 / 0	-84.3	-84.3	0.36 (1)		4.27
G- H	-3245 / 0	-84.3	-84.3	0.44 (1)		3.58
H- I	-3245 / 0	-84.3	-84.3	0.44 (1)		3.58
I- J	-4201 / 0	-84.3	-84.3	0.57 (1)		3.06
J- K	-3623 / 0	-84.3	-84.3	0.17 (1)		3.60
K- L	-2116 / 0	-84.3	-84.3	0.13 (1)		4.58
L- M	0 / 26	-84.3	-84.3	0.11 (1)		10.00
V- B	-341 / 0	0.0	0.0	0.02 (1)		7.81
N- L	-2199 / 0	0.0	0.0	0.14 (1)		6.88
V- U	0 / 2744	-28.0	-28.0	0.48 (2)		10.00
U- T	0 / 2443	-28.0	-28.0	0.45 (2)		10.00
T- S	0 / 2443	-28.0	-28.0	0.45 (2)		10.00
S- R	0 / 2602	-28.0	-28.0	0.45 (1)		10.00
R- Q	0 / 2602	-28.0	-28.0	0.45 (1)		10.00
Q- P	0 / 3230	-28.0	-28.0	0.59 (1)		10.00
P- O	0 / 1812	-28.0	-28.0	0.38 (1)		10.00
O- N	0 / 0	-28.0	-28.0	0.06 (1)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- U	-220 / 98	0.09 (1)
U- E	0 / 509	0.11 (2)
E- S	-763 / 0	0.38 (1)
S- F	0 / 1603	0.36 (1)
S- G	-1019 / 0	0.51 (1)
G- Q	0 / 826	0.19 (1)
Q- I	-578 / 0	0.23 (1)
I- P	0 / 686	0.15 (1)
P- J	-2138 / 0	0.31 (1)
P- K	0 / 2401	0.54 (1)
O- K	-704 / 0	0.10 (1)
O- L	0 / 2127	0.48 (1)
V- C	-3227 / 0	0.86 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.21")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.22")
ALLOWABLE DEFL.(TL)= L/360 (1.21")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.36")

CSI: TC=0.57 (I-J:1) , BC=0.59 (P-Q:1) , WB=0.86 (C-V:1) , SSI=0.23 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (U) (INPUT = 0.90)
JSI METAL= 0.79 (C) (INPUT = 1.00)

DWG NO. TAM 41942-17

STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285473	T106	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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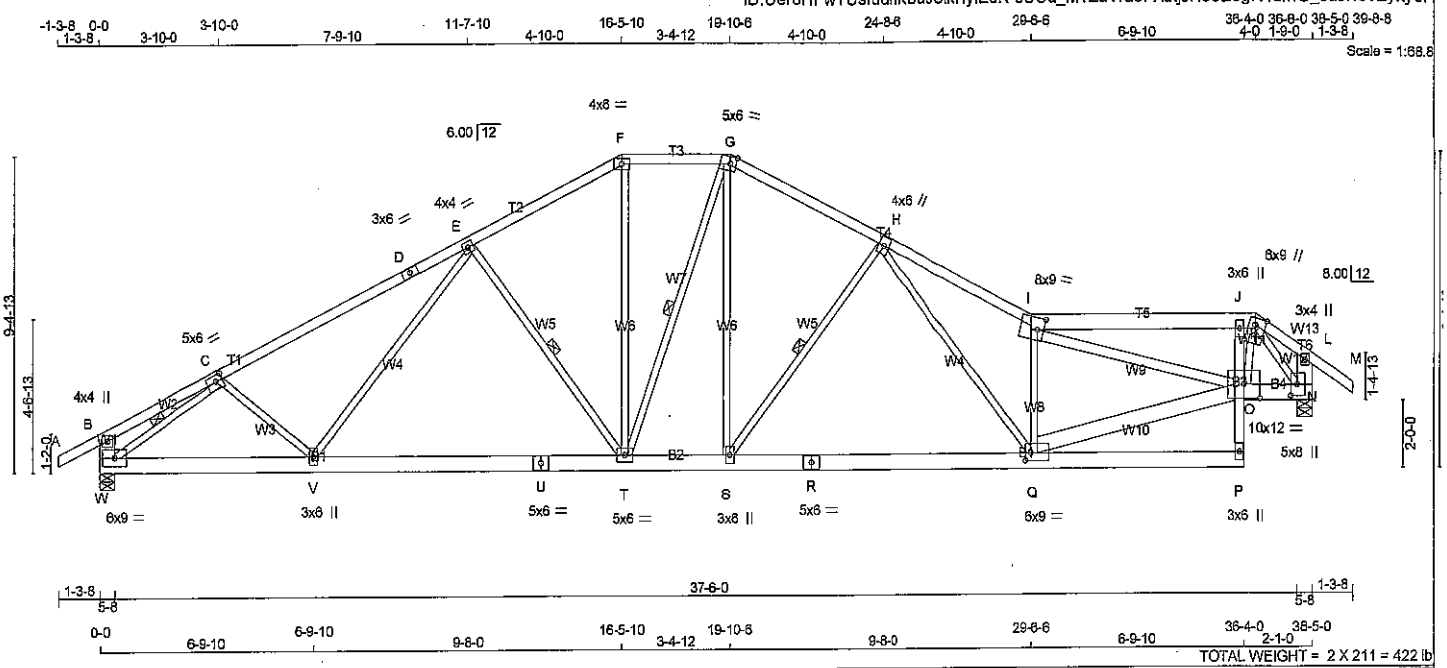


TOTAL WEIGHT = 2 X 211 = 422 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - K	2x6	DRY	No.2	SPF
K - M	2x4	DRY	No.2	SPF
W - B	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
W - U	2x6	DRY	No.2	SPF
U - R	2x6	DRY	No.2	SPF
R - P	2x6	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF
O - N	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
Q - O	2x6	DRY	No:2	SPF
I - O	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.25	2.50
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTW-I	MT20	4.0	6.0		
G	TTWW-m	MT20	5.0	6.0	2.50	2.25
H	TMWW+t	MT20	4.0	6.0		
I	TTWW-m	MT20	8.0	9.0	4.25	2.25
J	TMV+p	MT20	3.0	6.0		
K	TTWW+m	MT20	6.0	9.0	Edge	3.75
L	TMV+p	MT20	3.0	4.0		
N	BMVW1+p	MT20	5.0	6.0	4.00	2.25
O	BVMWWW-I	MT20	10.0	12.0	5.00	5.75
P	BMV+p	MT20	3.0	6.0		
Q	BMWWW-t	MT20	6.0	9.0	2.75	2.25
R	BS-t	MT20	5.0	6.0		
S	BMWW+t	MT20	3.0	6.0		
T	BMWWW-t	MT20	5.0	6.0		
U	BS-t	MT20	5.0	6.0		
V	BMWW+t	MT20	3.0	6.0		
W	BMVW1-t	MT20	6.0	9.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
W	2266	0	2266	0	0	5-8	5-8
N	2274	0	2274	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	1847	1051 / 0	403 / 0	0 / 0	0 / 0	392 / 0	0 / 0
N	1850	1054 / 0	403 / 0	0 / 0	0 / 0	393 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.17 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-T, G-T, H-S, C-W.

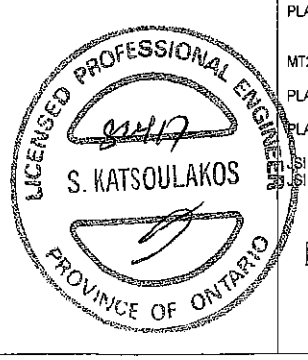
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 26	-84.3	-84.3	0.11 (1)	10.00
B-C	0 / 39	-84.3	-84.3	0.45 (1)	10.00
C-D	-3180 / 0	-84.3	-84.3	0.88 (1)	3.31
D-E	-3180 / 0	-84.3	-84.3	0.88 (1)	3.31
E-F	-2565 / 0	-84.3	-84.3	0.51 (1)	3.80
F-G	-2271 / 0	-84.3	-84.3	0.18 (1)	4.39
G-H	-2591 / 0	-84.3	-84.3	0.36 (1)	3.99
H-I	-3987 / 0	-84.3	-84.3	0.50 (1)	3.17
I-J	-2181 / 0	-84.3	-84.3	0.26 (1)	5.25
J-K	-2229 / 0	-84.3	-84.3	0.12 (1)	5.36
K-L	-25 / 0	-84.3	-84.3	0.08 (1)	6.25
L-M	0 / 32	-84.3	-84.3	0.11 (1)	10.00
W-B	-185 / 0	0.0	0.0	0.01 (1)	7.81
N-L	-235 / 0	0.0	0.0	0.02 (1)	7.81
W-V	0 / 2835	-28.0	-28.0	0.44 (2)	10.00
V-U	0 / 2681	-28.0	-28.0	0.45 (1)	10.00
U-T	0 / 2681	-28.0	-28.0	0.45 (1)	10.00
T-S	0 / 2307	-28.0	-28.0	0.41 (2)	10.00
S-R	0 / 2884	-28.0	-28.0	0.47 (1)	10.00
R-Q	0 / 2884	-28.0	-28.0	0.47 (1)	10.00
Q-P	0 / 24	-28.0	-28.0	0.22 (3)	10.00
P-O	0 / 116	0.0	0.0	0.08 (1)	10.00
O-J	-870 / 0	0.0	0.0	0.06 (1)	7.81
O-N	0 / 1645	-28.0	-28.0	0.21 (1)	10.00

WEBS FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-V	0 / 227	0.05 (3)
V-E	0 / 400	0.09 (2)
E-T	-701 / 0	0.32 (1)
T-F	0 / 867	0.20 (1)
T-G	-105 / 0	0.07 (1)
S-G	0 / 954	0.21 (1)
S-H	-981 / 0	0.45 (1)
H-Q	0 / 1166	0.26 (1)
Q-I	-1762 / 0	0.47 (1)
Q-O	0 / 3694	0.44 (1)
I-O	-1450 / 0	0.90 (1)
W-C	-3472 / 0	0.83 (1)
O-K	0 / 2551	0.57 (1)
K-N	-2596 / 0	0.43 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.28")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.24")
ALLOWABLE DEFL.(TL)= L/360 (1.28")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.40")

CSI: TC=0.88 (C-E:1) , BC=0.47 (Q-S:1) , WB=0.90 (I-O:1) , SSI=0.26 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1567	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.90 (C) (INPUT = 0.90)
SI METAL= 0.86 (C) (INPUT = 1.00)

DWG NO. TAM 41943-17
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285473	T107	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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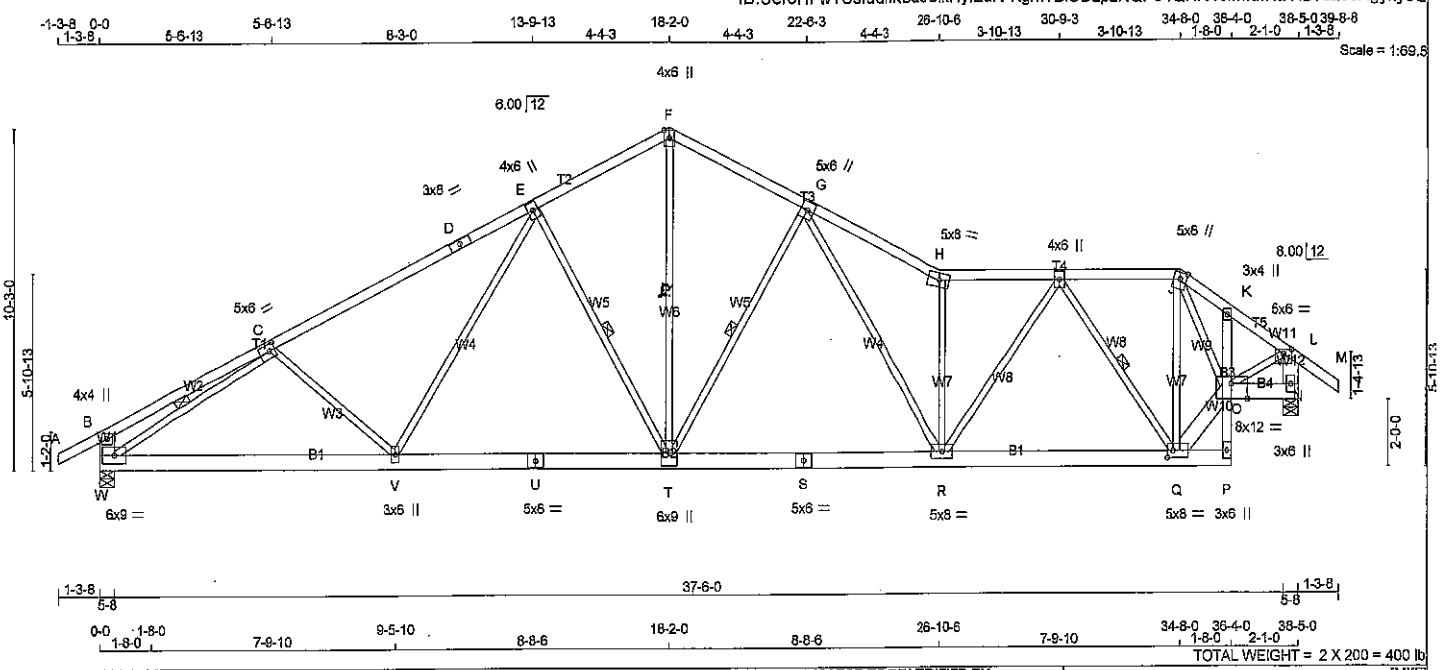


TOTAL WEIGHT = 2 X 200 = 400 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF
W - B	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
W - U	2x6	DRY	No.2	SPF
U - S	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - K	2x4	DRY	No.2	SPF
O - N	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
Q - O	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.25
D	TS-t	MT20	3.0	8.0		
E	TMWW+t	MT20	4.0	6.0		
F	TTW+p	MT20	4.0	6.0	Edge	
G	TMWW+t	MT20	5.0	6.0		
H	TTW-m	MT20	5.0	8.0		
I	TMWW+t	MT20	4.0	6.0		
J	TTWW+m	MT20	5.0	6.0	Edge	
K	TMV+p	MT20	3.0	4.0		
L	TMVW-p	MT20	5.0	6.0	1.50	3.00
N	BMV1+p	MT20	3.0	6.0		
O	BVMWWW-I	MT20	8.0	12.0	Edge	6.00
P	BMV+p	MT20	3.0	6.0		
Q	BMWWW-t	MT20	5.0	6.0	2.50	2.25
R	BMWWW-t	MT20	5.0	6.0		
S	BS-t	MT20	5.0	6.0		
T	BMWWW+t	MT20	6.0	9.0		
U	BS-t	MT20	5.0	6.0		
V	BMWW+t	MT20	3.0	6.0		
W	BMVW1-t	MT20	6.0	9.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	2272	0	2272	0	0	5-8	5-8
W	2271	0	2271	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1849	1053 / 0	403 / 0	0 / 0	0 / 0	392 / 0	0 / 0
W	1848	1052 / 0	403 / 0	0 / 0	0 / 0	392 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, W

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.28 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

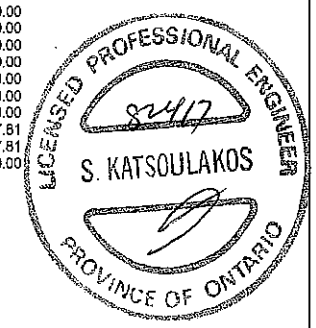
1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-T, G-T, I-Q, C-W. F-T

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 26	-84.3	-84.3	1	0.11 (1)	10.00	C-V	-236 / 133	0.11 (1)
B-C	0 / 34	-84.3	-84.3	1	0.57 (1)	10.00	V-E	0 / 538	0.12 (2)
C-D	-3090 / 0	-84.3	-84.3	1	0.98 (1)	3.28	E-T	-828 / 0	0.44 (1)
D-E	-3090 / 0	-84.3	-84.3	1	0.98 (1)	3.28	T-F	0 / 1882	0.42 (1)
E-F	-2405 / 0	-84.3	-84.3	1	0.50 (1)	3.91	T-G	-1028 / 0	0.54 (1)
F-G	-2384 / 0	-84.3	-84.3	1	0.28 (1)	4.22	G-R	0 / 1117	0.25 (1)
G-H	-3535 / 0	-84.3	-84.3	1	0.37 (1)	3.47	R-H	-1854 / 0	0.93 (1)
H-I	-3147 / 0	-84.3	-84.3	1	0.28 (1)	3.73	R-I	0 / 1312	0.30 (1)
I-J	-1357 / 0	-84.3	-84.3	1	0.19 (1)	5.33	I-Q	-1796 / 0	0.55 (1)
J-K	-2155 / 0	-84.3	-84.3	1	0.10 (1)	4.58	Q-J	0 / 152	0.03 (3)
K-L	-2190 / 0	-84.3	-84.3	1	0.12 (1)	4.52	Q-O	0 / 2160	0.25 (1)
L-M	0 / 32	-84.3	-84.3	1	0.11 (1)	10.00	J-O	0 / 1022	0.23 (1)
W-B	-269 / 0	0.0	0.0	1	0.02 (1)	7.81	O-L	0 / 1990	0.45 (1)
N-L	-2241 / 0	0.0	0.0	1	0.15 (1)	6.84	W-C	-3493 / 0	0.97 (1)
W-V	0 / 2940	-28.0	-28.0	1	0.51 (1)	10.00			
V-U	0 / 2524	-28.0	-28.0	1	0.46 (2)	10.00			
U-T	0 / 2524	-28.0	-28.0	1	0.46 (2)	10.00			
T-S	0 / 2623	-28.0	-28.0	1	0.45 (1)	10.00			
S-R	0 / 2623	-28.0	-28.0	1	0.45 (1)	10.00			
R-Q	0 / 2404	-28.0	-28.0	1	0.41 (1)	10.00			
Q-P	0 / 24	-28.0	-28.0	1	0.15 (2)	10.00			
P-O	-100 / 0	0.0	0.0	1	0.05 (1)	7.81			
O-K	-133 / 11	0.0	0.0	1	0.05 (1)	7.81			
O-N	0 / 0	-28.0	-28.0	2	0.02 (2)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.28")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.23")
ALLOWABLE DEFL.(TL)= L/360 (1.28")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.39")

CSI: TC=0.98 (C-E:1) , BC=0.51 (V-W:1) , WB=0.97 (C-W:1) , SSI=0.25 (C-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1557	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.85 (C) (INPUT = 1.00)

DWG NO. TAM 41947-17

STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285473	T108	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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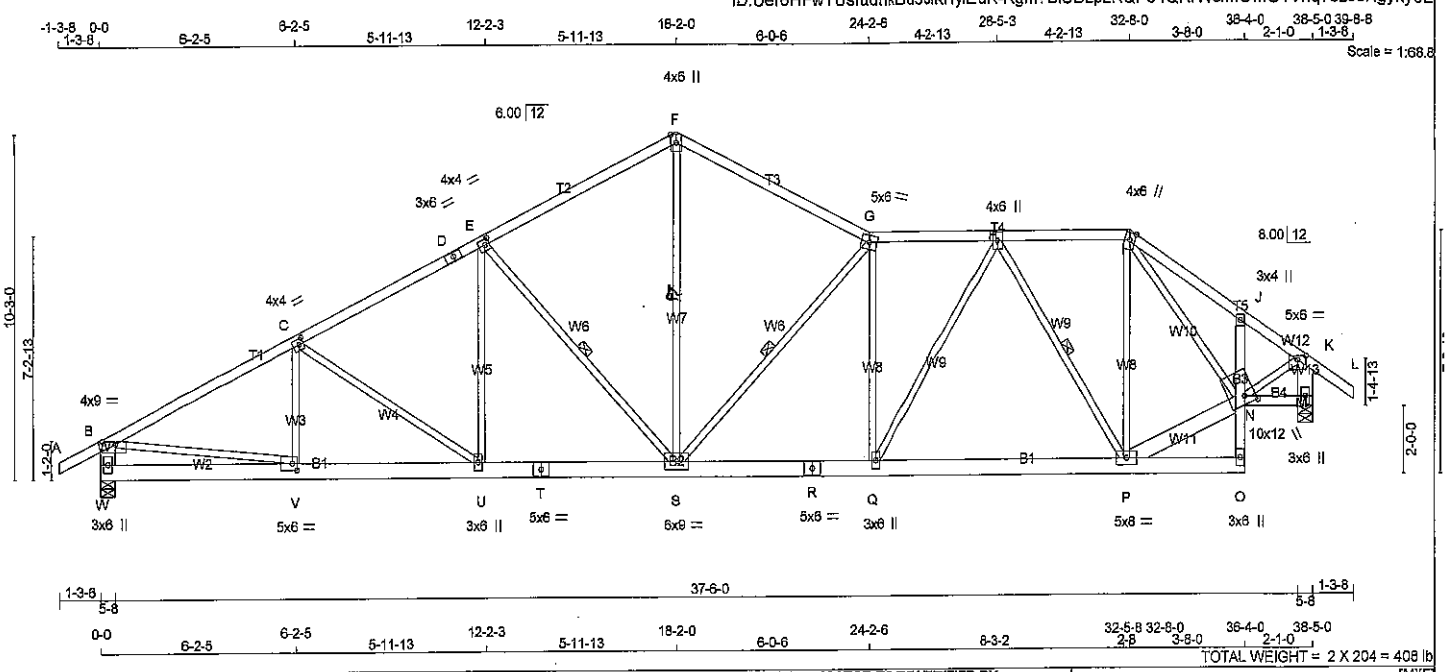


[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
W - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
W - T	2x8	DRY	No.2	SPF
T - R	2x6	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
O - J	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	9.0	1.00	4.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.75
F	TTW+p	MT20	4.0	6.0	Edge	
G	TTWW-m	MT20	5.0	6.0		
H	TMWW+t	MT20	4.0	6.0		
I	TTWW+m	MT20	4.0	6.0	Edge	
J	TMV+p	MT20	3.0	4.0		
K	TMVW-p	MT20	5.0	6.0	1.50	3.00
M	BMV1+p	MT20	3.0	6.0		
N	BVMWWW-w	MT20	10.0	12.0	3.75	3.00
O	BMV+p	MT20	3.0	6.0		
P	BMWWW-t	MT20	5.0	8.0		
Q	BMWW+t	MT20	3.0	6.0		
R	BS-t	MT20	5.0	6.0		
S	BMWWW-t	MT20	6.0	9.0		
T	BS-t	MT20	5.0	6.0		
U	BMWW+t	MT20	3.0	6.0		
V	BMWW-t	MT20	5.0	6.0	2.50	1.75
W	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
W	2271	0	2271	0	0	5-8	5-8
M	2272	0	2272	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	1848	1052 / 0	403 / 0	0 / 0	0 / 0	392 / 0	0 / 0
M	1849	1053 / 0	403 / 0	0 / 0	0 / 0	392 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.37 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-S, G-S, H-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

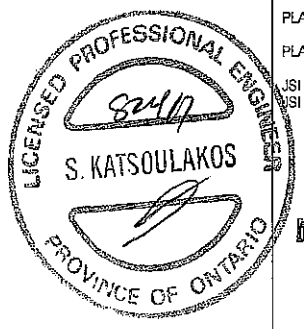
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 26	-84.3	-84.3	0.11 (1)	10.00
B- C	-3295 / 0	-84.3	-84.3	0.64 (1)	3.37
C- D	-2990 / 0	-84.3	-84.3	0.50 (1)	3.65
D- E	-2990 / 0	-84.3	-84.3	0.50 (1)	3.65
E- F	-2373 / 0	-84.3	-84.3	0.49 (1)	4.01
F- G	-2350 / 0	-84.3	-84.3	0.51 (1)	3.99
G- H	-2819 / 0	-84.3	-84.3	0.29 (1)	3.91
H- I	-1584 / 0	-84.3	-84.3	0.22 (1)	5.04
I- J	-2142 / 0	-84.3	-84.3	0.19 (1)	4.50
J- K	-2139 / 0	-84.3	-84.3	0.12 (1)	4.57
K- L	0 / 32	-84.3	-84.3	0.11 (1)	10.00
W- B	-2183 / 0	0.0	0.0	0.14 (1)	6.90
M- K	-2241 / 0	0.0	0.0	0.15 (1)	6.84
W- V	0 / 0	-28.0	-28.0	0.12 (2)	10.00
V- U	0 / 2966	-28.0	-28.0	0.41 (1)	10.00
U- T	0 / 2675	-28.0	-28.0	0.37 (1)	10.00
T- S	0 / 2675	-28.0	-28.0	0.37 (1)	10.00
S- R	0 / 2828	-28.0	-28.0	0.39 (1)	10.00
R- Q	0 / 2828	-28.0	-28.0	0.39 (1)	10.00
Q- P	0 / 2331	-28.0	-28.0	0.38 (1)	10.00
P- O	0 / 19	-28.0	-28.0	0.17 (2)	10.00
O- N	0 / 22	0.0	0.0	0.05 (1)	10.00
N- J	-283 / 0	0.0	0.0	0.04 (1)	7.81
N- M	0 / 0	-28.0	-28.0	0.04 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V- C	-213 / 130	0.05 (1)
C- U	-349 / 0	0.32 (1)
U- E	0 / 438	0.10 (2)
E- S	-853 / 0	0.49 (1)
S- F	0 / 1635	0.37 (1)
S- G	-1097 / 0	0.63 (1)
Q- G	-621 / 0	0.53 (1)
P- I	0 / 639	0.14 (1)
P- N	0 / 1760	0.21 (1)
I- N	0 / 337	0.08 (1)
B- V	0 / 2989	0.67 (1)
N- K	0 / 1993	0.45 (1)
Q- H	0 / 945	0.21 (1)
H- P	-1448 / 0	0.62 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.28")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.19")
ALLOWABLE DEFL.(TL)= L/360 (1.28")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.31")

CSI: TC=0.64 (B-C:1) , BC=0.41 (U-V:1) , WB=0.67 (B-V:1) , SSI=0.21 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.74 (V) (INPUT = 1.00)

DWG NO. TAM 41945-17
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285473	T109	6	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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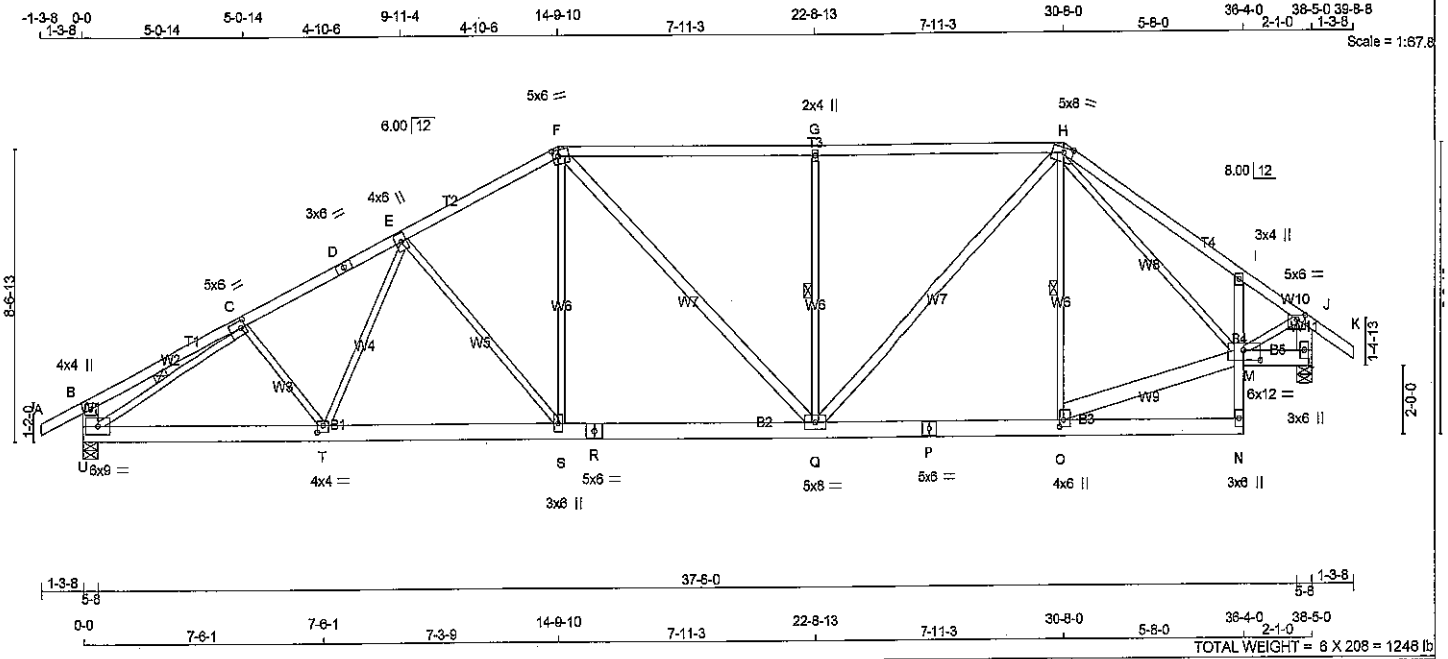


[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
U - R	2x6	DRY	No.2	SPF
R - P	2x6	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
N - I	2x4	DRY	No.2	SPF
M - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
F - Q	2x4	DRY	No.2	SPF
Q - H	2x4	DRY	No.2	SPF
O - M	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	1.75
D	TS-t	MT20	3.0	6.0		
E	TMWW+t	MT20	4.0	6.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMW+w	MT20	2.0	4.0		
H	TTWWW-m	MT20	5.0	8.0	1.75	3.50
I	TMV+p	MT20	3.0	4.0		
J	TMVW-p	MT20	5.0	6.0	1.50	3.00
L	BMV1+p	MT20	3.0	6.0		
M	BVMWWW-I	MT20	6.0	12.0	3.50	6.00
N	BMV+p	MT20	3.0	8.0		
O	BMWW+t	MT20	4.0	6.0	2.50	1.50
P	BS-t	MT20	5.0	6.0		
Q	BMWWW-t	MT20	5.0	8.0		
R	BS-t	MT20	5.0	6.0		
S	BMWW+t	MT20	3.0	6.0		
T	BMWW-t	MT20	4.0	4.0	2.50	2.00
U	BMVW1-t	MT20	6.0	9.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	2272	0	2272	0	0	5-8	5-8
U	2271	0	2271	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1849	1053 / 0	403 / 0	0 / 0	0 / 0	392 / 0	0 / 0
U	1848	1052 / 0	403 / 0	0 / 0	0 / 0	392 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, U

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-Q, H-O, C-U.

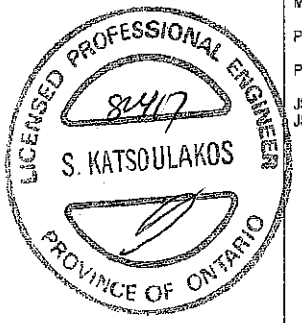
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 26	-84.3	-84.3	0.11 (1)	10.00
B- C	0 / 16	-84.3	-84.3	0.25 (1)	10.00
C- D	-3211 / 0	-84.3	-84.3	0.37 (1)	3.64
D- E	-3211 / 0	-84.3	-84.3	0.37 (1)	3.64
E- F	-2711 / 0	-84.3	-84.3	0.34 (1)	3.95
F- G	-2534 / 0	-84.3	-84.3	0.89 (1)	3.20
G- H	-2534 / 0	-84.3	-84.3	0.89 (1)	3.20
H- I	-2276 / 0	-84.3	-84.3	0.43 (1)	4.15
I- J	-2248 / 0	-84.3	-84.3	0.28 (1)	4.30
J- K	0 / 32	-84.3	-84.3	0.11 (1)	10.00
U- B	-289 / 0	0.0	0.0	0.02 (1)	7.81
L- J	-2241 / 0	0.0	0.0	0.15 (1)	6.84
U- T	0 / 2875	-28.0	-28.0	0.45 (1)	10.00
T- S	0 / 2786	-28.0	-28.0	0.42 (1)	10.00
S- R	0 / 2415	-28.0	-28.0	0.38 (1)	10.00
R- Q	0 / 2415	-28.0	-28.0	0.38 (1)	10.00
Q- P	0 / 1652	-28.0	-28.0	0.30 (2)	10.00
P- O	0 / 1652	-28.0	-28.0	0.30 (2)	10.00
O- N	0 / 34	-28.0	-28.0	0.16 (2)	10.00
N- M	0 / 97	0.0	0.0	0.10 (1)	10.00
M- I	-442 / 0	0.0	0.0	0.08 (1)	7.81
M- L	0 / 0	-28.0	-28.0	0.02 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- T	-1 / 166	0.04 (3)
T- E	0 / 310	0.07 (2)
E- S	-579 / 0	0.66 (1)
S- F	0 / 665	0.15 (1)
F- Q	0 / 171	0.03 (1)
Q- G	-825 / 0	0.37 (1)
Q- H	0 / 1267	0.20 (1)
O- H	-354 / 70	0.16 (1)
O- M	0 / 1722	0.20 (1)
H- M	0 / 327	0.07 (1)
U- C	-3438 / 0	0.83 (1)
M- J	0 / 2099	0.47 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.28")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (1.28")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.27")

CSI: TC=0.89 (F-G:1) , BC=0.45 (T-U:1) , WB=0.83 (C-U:1) , SSI=0.32 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (F) (INPUT = 0.90)
JSI METAL= 0.85 (C) (INPUT = 1.00)

DWG NO. TAM 41996-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285473	T110	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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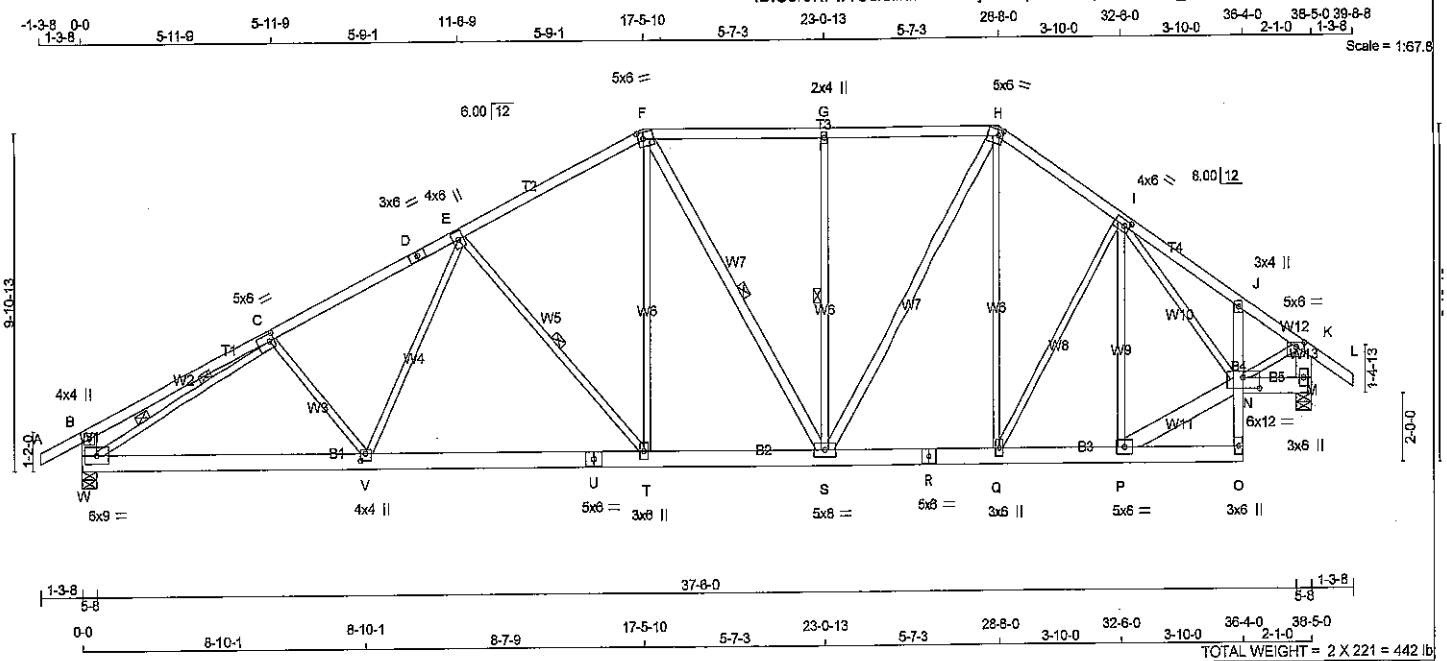


TOTAL WEIGHT = 2 X 221 = 442 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - L	2x4	DRY	No.2	SPF
W - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
W - U	2x6	DRY	No.2	SPF
U - R	2x6	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
O - J	2x4	DRY	No.2	SPF
N - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
F - S	2x4	DRY	No.2	SPF
S - H	2x4	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	1.75
D	TS-t	MT20	3.0	6.0		
E	TMWW+t	MT20	4.0	6.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	6.0	2.00	1.75
I	TMWWW-t	MT20	4.0	6.0	2.00	2.00
J	TMV+p	MT20	3.0	4.0		
K	TMVW-p	MT20	5.0	6.0	1.50	3.00
M	BMV1+p	MT20	3.0	6.0		
N	BVMWWW-t	MT20	6.0	12.0	3.75	6.00
O	BMV+p	MT20	3.0	6.0		
P	BMWW-t	MT20	5.0	6.0		
Q	BMWW+t	MT20	3.0	6.0		
R	BS-t	MT20	5.0	6.0		
S	BMWWW-t	MT20	5.0	8.0		
T	BMWW+t	MT20	3.0	6.0		
U	BS-t	MT20	5.0	6.0		
V	BMWW+t	MT20	4.0	4.0	2.50	1.75
W	BMVW1-t	MT20	6.0	9.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
M	2272	0	2272	0	0	5-8	5-8
W	2271	0	2271	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1849	1053 / 0	403 / 0	0 / 0	0 / 0	392 / 0	0 / 0
W	1848	1052 / 0	403 / 0	0 / 0	0 / 0	392 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, W

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.56 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

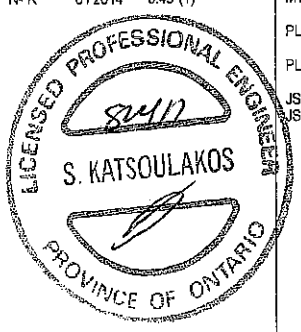
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-T, F-S, G-S.
2 LATERAL BRACE(S) AT 1/ 3 LENGTH OF C-W.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
A- B	0 / 26	-84.3	-84.3	0.11 (1)	10.00	C- V	-138 / 131	0.05 (1)	
B- C	0 / 20	-84.3	-84.3	0.36 (1)	10.00	V- E	0 / 479	0.11 (2)	
C- D	-3170 / 0	-84.3	-84.3	0.50 (1)	3.56	E- T	-774 / 0	0.41 (1)	
D- E	-3170 / 0	-84.3	-84.3	0.50 (1)	3.56	T- F	0 / 768	0.18 (1)	
E- F	-2441 / 0	-84.3	-84.3	0.44 (1)	4.02	F- S	-103 / 0	0.07 (2)	
F- G	-2124 / 0	-84.3	-84.3	0.40 (1)	4.27	S- G	-576 / 0	0.37 (1)	
G- H	-2124 / 0	-84.3	-84.3	0.40 (1)	4.27	S- H	0 / 926	0.15 (1)	
H- I	-2013 / 0	-84.3	-84.3	0.20 (1)	4.60	Q- H	0 / 154	0.03 (3)	
I- J	-2156 / 0	-84.3	-84.3	0.21 (1)	4.47	Q- I	0 / 138	0.03 (1)	
J- K	-2202 / 0	-84.3	-84.3	0.09 (1)	4.54	P- I	-710 / 0	0.64 (1)	
K- L	0 / 32	-84.3	-84.3	0.11 (1)	10.00	P- N	0 / 1773	0.21 (1)	
W- B	-317 / 0	0.0	0.0	0.02 (1)	7.81	I- N	0 / 372	0.08 (1)	
M- K	-2241 / 0	0.0	0.0	0.15 (1)	6.84	W- C	-3457 / 0	1.00 (1)	
						N- K	0 / 2014	0.45 (1)	
W- V	0 / 2922	-28.0	-28.0	0.49 (2)	10.00				
V- U	0 / 2675	-28.0	-28.0	0.47 (2)	10.00				
U- T	0 / 2675	-28.0	-28.0	0.47 (2)	10.00				
T- S	0 / 2172	-28.0	-28.0	0.34 (1)	10.00				
S- R	0 / 1654	-28.0	-28.0	0.24 (1)	10.00				
R- Q	0 / 1654	-28.0	-28.0	0.24 (1)	10.00				
Q- P	0 / 1586	-28.0	-28.0	0.24 (1)	10.00				
P- O	0 / 28	-28.0	-28.0	0.06 (2)	10.00				
O- N	0 / 70	0.0	0.0	0.08 (1)	10.00				
N- J	-212 / 0	0.0	0.0	0.06 (1)	7.81				
N- M	0 / 0	-28.0	-28.0	0.02 (2)	10.00				



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.28")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (1.28")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.28")

CSI: TC=0.50 (C-E:1) , BC=0.49 (V-W:2) , WB=1.00 (C-W:1) , SSI=0.23 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (H) (INPUT = 0.90)
JSI METAL= 0.85 (C) (INPUT = 1.00)

DWG NO. TAM 41847-17
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285473	T111	6	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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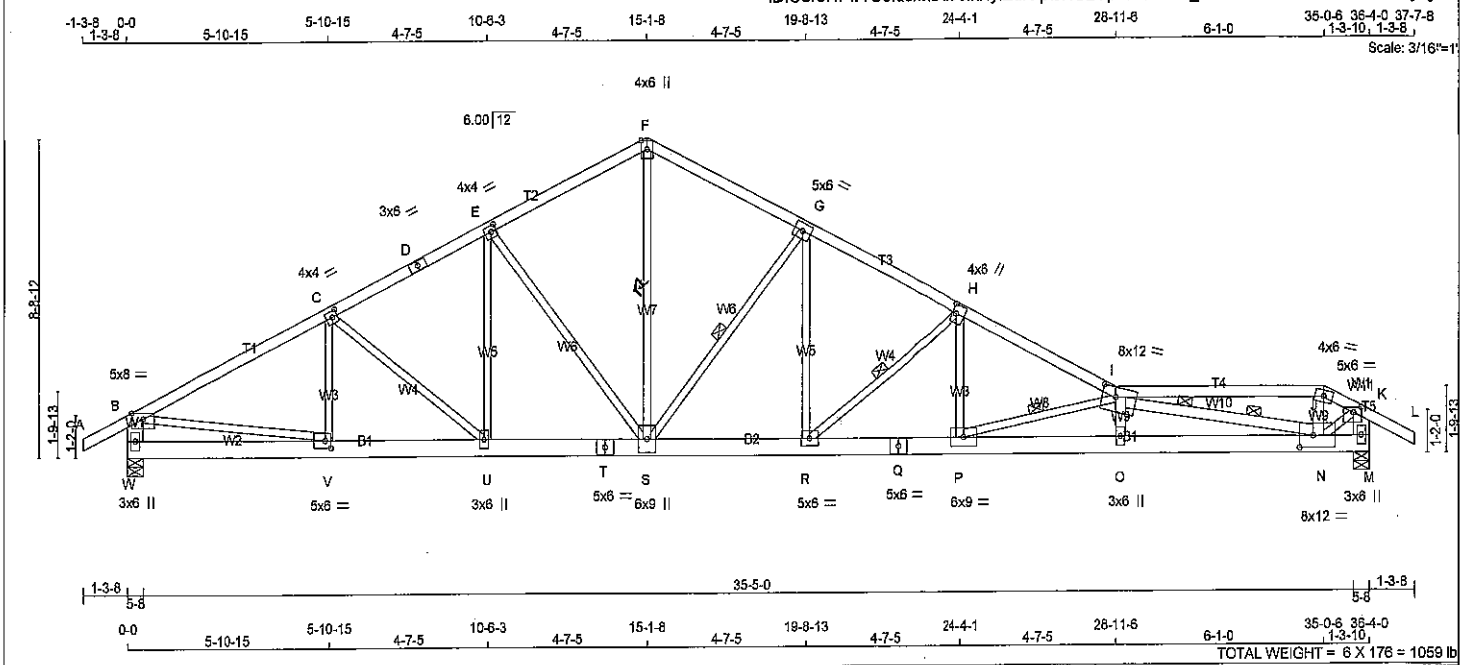


TOTAL WEIGHT = 6 X 176 = 1059 lb

[M|F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
I - J	2x4	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
W - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
W - T	2x6	DRY	1650F 1.5E	SPF
T - Q	2x6	DRY	1650F 1.5E	SPF
Q - M	2x6	DRY	1650F 1.5E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
O - I	2x4	DRY	No.2	SPF
I - N	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.75
F	TTW+p	MT20	4.0	6.0	Edge	
G	TMWW-t	MT20	5.0	6.0		
H	TMWW-t	MT20	4.0	6.0	3.00	1.25
I	TTWWW-m	MT20	8.0	12.0	3.25	4.75
J	TTW-m	MT20	4.0	6.0		
K	TMVW-p	MT20	5.0	6.0	1.75	2.75
M	BMV1+p	MT20	3.0	6.0		
N	BMWWW-t	MT20	8.0	12.0	4.00	4.75
O	BMW+w	MT20	3.0	6.0		
P	BMWW-t	MT20	6.0	9.0		
Q	BS-t	MT20	5.0	8.0		
R	BMWW-t	MT20	5.0	6.0		
S	BMWWW+t	MT20	6.0	9.0		
T	BS-t	MT20	5.0	6.0		
U	BMWW+t	MT20	3.0	6.0		
V	BMWW-t	MT20	5.0	6.0	2.50	2.00
W	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
W	2151	0	2151	0	0	5-8	5-8
M	2156	0	2156	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	1751	998 / 0	382 / 0	0 / 0	0 / 0	371 / 0	0 / 0
M	1753	1000 / 0	382 / 0	0 / 0	0 / 0	372 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.80 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

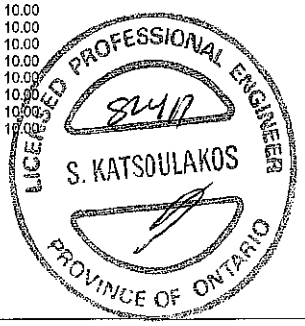
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-S, H-R, I-P.
2 LATERAL BRACE(S) AT 1/ 3 LENGTH OF I-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO					FR-TO			
A- B	0 / 26	-84.3	-84.3	0.11 (1)	10.00	V- C	-223 / 104	0.05 (1)
B- C	-3069 / 0	-84.3	-84.3	0.55 (1)	3.54	C- U	-278 / 0	0.17 (1)
C- D	-2855 / 0	-84.3	-84.3	0.38 (1)	3.80	U- E	0 / 327	0.07 (2)
D- E	-2855 / 0	-84.3	-84.3	0.38 (1)	3.80	E- S	-622 / 0	0.70 (1)
E- F	-2438 / 0	-84.3	-84.3	0.30 (1)	4.16	S- F	0 / 1647	0.42 (1)
F- G	-2440 / 0	-84.3	-84.3	0.28 (1)	4.17	S- G	-1467 / 0	0.56 (1)
G- H	-3419 / 0	-84.3	-84.3	0.35 (1)	3.58	R- G	0 / 1186	0.27 (1)
H- I	-4970 / 0	-84.3	-84.3	0.56 (1)	2.80	R- H	-1807 / 0	0.45 (1)
I- J	-2154 / 0	-84.3	-84.3	0.46 (1)	4.14	P- H	0 / 1408	0.32 (1)
J- K	-2405 / 0	-84.3	-84.3	0.14 (1)	4.33	P- I	-3529 / 0	0.72 (1)
K- L	0 / 26	-84.3	-84.3	0.11 (1)	10.00	O- I	-245 / 69	0.03 (1)
W- B	-2065 / 0	0.0	0.0	0.13 (1)	7.05	I- N	-5812 / 0	0.73 (1)
M- K	-2512 / 0	0.0	0.0	0.16 (1)	6.53	N- J	0 / 824	0.13 (1)
						B- V	0 / 2765	0.83 (1)
W- V	0 / 0	-28.0	-28.0	0.08 (2)	10.00	N- K	0 / 2441	0.55 (1)
V- U	0 / 2762	-28.0	-28.0	0.25 (1)	10.00			
U- T	0 / 2545	-28.0	-28.0	0.24 (1)	10.00			
T- S	0 / 2545	-28.0	-28.0	0.24 (1)	10.00			
S- R	0 / 3058	-28.0	-28.0	0.27 (1)	10.00			
R- Q	0 / 4463	-28.0	-28.0	0.40 (1)	10.00			
Q- P	0 / 4463	-28.0	-28.0	0.40 (1)	10.00			
P- O	0 / 7833	-28.0	-28.0	0.91 (1)	10.00			
O- N	0 / 7808	-28.0	-28.0	0.91 (1)	10.00			
N- M	0 / 0	-28.0	-28.0	0.24 (1)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.21")
CALCULATED VERT. DEFL.(LL) = L/ 810 (0.54")
ALLOWABLE DEFL.(TL)= L/360 (1.21")
CALCULATED VERT. DEFL.(TL) = L/ 504 (0.86")

CSI: TC=0.56 (H-I:1) , BC=0.91 (O-P:1) , WB=0.73 (I-N:1) , SSI=0.24 (M-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (P) (INPUT = 0.90)
JSI METAL= 0.99 (I) (INPUT = 1.00)

DWG NO. TAM41948-17
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285479	T112	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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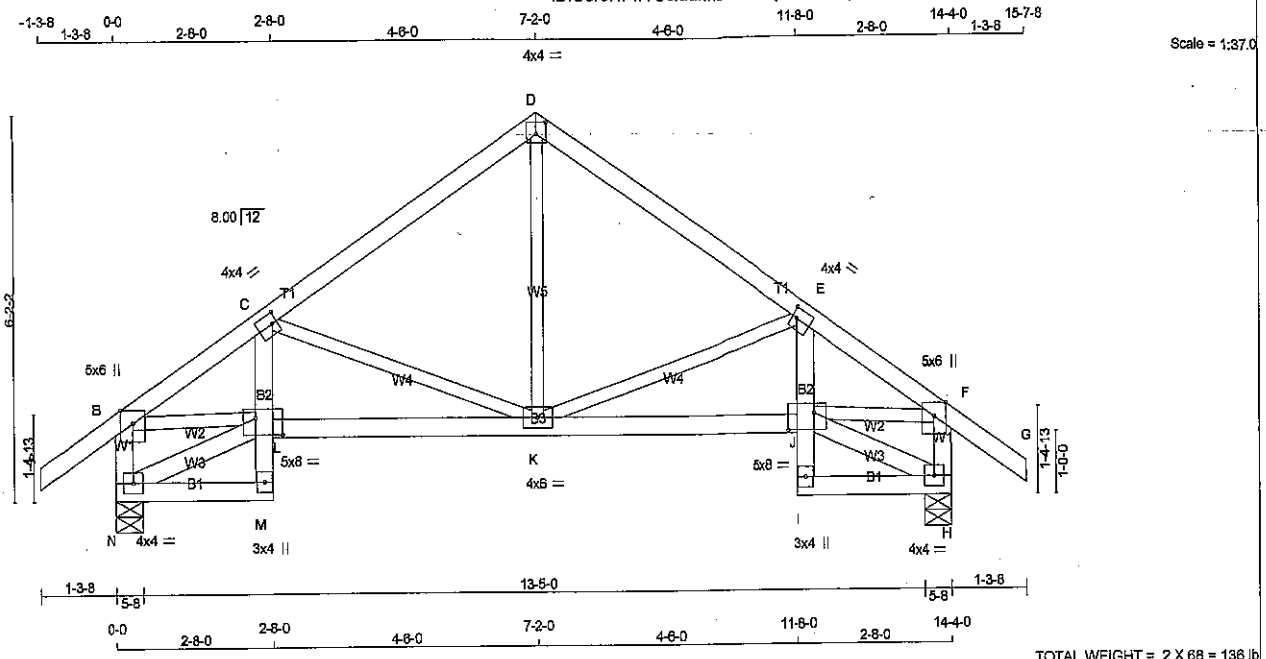

TOTAL WEIGHT = 2 X 68 = 136 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
M - C	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
I - E	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMVW-t	MT20	4.0	4.0	2.00	1.00
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMVW-t	MT20	4.0	4.0	2.00	1.00
F	TMVW+p	MT20	5.0	6.0	Edge	
H	BMVW1-t	MT20	4.0	4.0		
I	BMV+p	MT20	3.0	4.0		
J	BVMWW-I	MT20	5.0	8.0	3.25	5.50
K	BMWWW-t	MT20	4.0	6.0		
L	BVMWW-I	MT20	5.0	8.0	3.25	5.50
M	BMV+p	MT20	3.0	4.0		
N	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REORD BRG IN-SX
N	920	0	920	0	0	5-8	5-8
H	920	0	920	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	739	437 / 0	150 / 0	0 / 0	0 / 0	152 / 0	0 / 0
H	739	437 / 0	150 / 0	0 / 0	0 / 0	152 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.60 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FR-TO	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	FR-TO	MAX CSI (LC)
A- B	0 / 32		-84.3	-84.3	0.11 (1)	10.00		K- D	0 / 482		0.11 (1)
B- C	-1234 / 0		-84.3	-84.3	0.17 (1)	5.60		K- E	-500 / 0		0.20 (1)
C- D	-748 / 0		-84.3	-84.3	0.22 (1)	6.25		C- K	-500 / 0		0.20 (1)
D- E	-748 / 0		-84.3	-84.3	0.22 (1)	6.25		N- L	-40 / 0		0.00 (1)
E- F	-1234 / 0		-84.3	-84.3	0.17 (1)	5.60		B- L	0 / 1053		0.24 (1)
F- G	0 / 32		-84.3	-84.3	0.11 (1)	10.00		J- H	-40 / 0		0.00 (1)
N- B	-869 / 0		0.0	0.0	0.09 (1)	7.81		J- F	0 / 1053		0.24 (1)
H- F	-869 / 0		0.0	0.0	0.09 (1)	7.81					
N- M	0 / 37		-28.0	-28.0	0.06 (2)	10.00					
M- L	0 / 62		0.0	0.0	0.06 (1)	10.00					
L- C	0 / 147		0.0	0.0	0.07 (1)	10.00					
L- K	0 / 1076		-28.0	-28.0	0.30 (2)	10.00					
K- J	0 / 1076		-28.0	-28.0	0.30 (2)	10.00					
I- J	0 / 62		0.0	0.0	0.06 (1)	10.00					
J- E	0 / 147		0.0	0.0	0.07 (1)	10.00					
I- H	0 / 37		-28.0	-28.0	0.06 (2)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.48")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.48")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.22/1.00 (D-E:1) , BC=0.30/1.00 (J-K:2) , WB=0.24/1.00 (F-J:1) , SSI=0.15/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	22B4	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (L) (INPUT = 0.90)
JSI METAL= 0.24 (F) (INPUT = 1.00)

DWG NO. TAM21166-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285473	T113	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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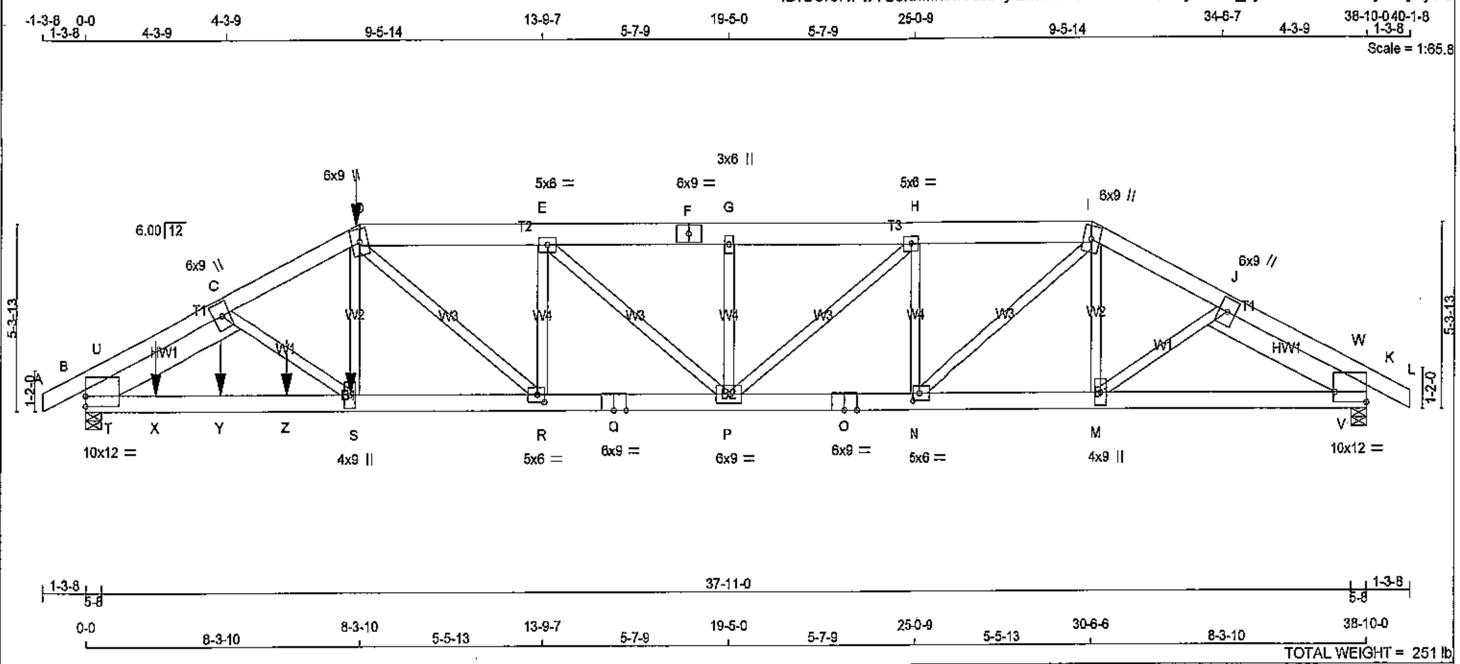

TOTAL WEIGHT = 251 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - F	2x8	DRY	No.2	SPF
F - I	2x8	DRY	No.2	SPF
I - L	2x6	DRY	No.2	SPF
B - Q	2x6	DRY	2100F 1.8E	SPF
Q - O	2x6	DRY	2100F 1.8E	SPF
O - K	2x6	DRY	2100F 1.8E	SPF

REINFORCING MEMBERS

HW1	2x8	DRY	No.2	SPF
HW2	2x8	DRY	No.2	SPF

ALL WEBS 2x4 DRY No.2 SPF
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMW1-I	MT20	10.0	12.0	3.50	
C	TMWW+t	MT20	6.0	9.0		
D	TTWW+m	MT20	6.0	9.0		
E	TMWW-t	MT20	5.0	6.0		
F	TS-t	MT20	6.0	9.0		
G	TMW+w	MT20	3.0	6.0		
H	TMWW-t	MT20	5.0	6.0		
I	TTWW+m	MT20	6.0	9.0		
J	TMWW+t	MT20	6.0	9.0		
K	TMBMW1-I	MT20	10.0	12.0	3.50	Edge
M	BMWW+t	MT20	4.0	9.0		
N	BMWW-t	MT20	5.0	6.0	2.50	2.50
O	BS-t	MT20	6.0	9.0		
P	BMWWW-t	MT20	6.0	9.0		
Q	BS-t	MT20	6.0	9.0		
R	BMWW-t	MT20	5.0	6.0	2.50	2.50
S	BMWW+t	MT20	4.0	9.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 499.5 lbs FACTORED DOWN AT 8-3-10 ON TOP CHORD, AND 71.7 lbs FACTORED DOWN AT 2-1-4, 71.7 lbs FACTORED DOWN AT 4-1-4, AND 71.7 lbs FACTORED DOWN AT 6-1-4, AND 2507.0 lbs FACTORED DOWN AT 8-0-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
B	4780	0	4780	0	0	5-8	5-8
K	2943	0	2943	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SDIL
B	3903	2202 / 0	865 / 0	0 / 0	0 / 0	835 / 0	0 / 0
K	2391	1369 / 0	517 / 0	0 / 0	0 / 0	505 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.72 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 0	-84.3	-84.3	0.06 (1)	10.00	C- S	0 / 707	0.13 (1)
B- U	-5758 / 0	-84.3	-84.3	0.23 (1)	3.47	S- D	0 / 2341	0.41 (1)
U- C	-3738 / 0	-84.3	-84.3	0.16 (1)	4.30	M- I	0 / 216	0.04 (3)
C- D	-7967 / 0	-84.3	-84.3	0.48 (1)	2.72	M- J	0 / 264	0.05 (2)
D- E	-7461 / 0	-84.3	-84.3	0.36 (1)	3.44	N- I	0 / 2455	0.43 (1)
E- F	-7060 / 0	-84.3	-84.3	0.32 (1)	3.56	D- R	0 / 515	0.09 (1)
F- G	-7060 / 0	-84.3	-84.3	0.32 (1)	3.56	N- H	-1478 / 0	0.38 (1)
G- H	-7060 / 0	-84.3	-84.3	0.31 (1)	3.56	R- E	-189 / 87	0.05 (1)
H- I	-5942 / 0	-84.3	-84.3	0.27 (1)	3.89	P- H	0 / 1467	0.26 (1)
I- J	-4578 / 0	-84.3	-84.3	0.22 (1)	3.88	E- P	-526 / 0	0.37 (1)
J- W	-2214 / 0	-84.3	-84.3	0.08 (1)	5.40	P- G	-458 / 0	0.12 (1)
W- K	-3406 / 0	-84.3	-84.3	0.11 (1)	4.52	T- U	0 / 2820	0.00 (1)
K- L	0 / 0	-84.3	-84.3	0.06 (1)	10.00	T- C	-4135 / 0	0.58 (1)
						J- V	-2444 / 0	0.34 (1)
B- T	0 / 3331	-28.0	-28.0	0.23 (2)	10.00	V- W	0 / 1599	0.00 (1)
T- X	0 / 6574	-28.0	-28.0	0.45 (1)	10.00			
X- Y	0 / 6574	-28.0	-28.0	0.45 (1)	10.00			
Y- Z	0 / 6574	-28.0	-28.0	0.45 (1)	10.00			
Z- S	0 / 6574	-28.0	-28.0	0.45 (1)	10.00			
S- R	0 / 7073	-28.0	-28.0	0.40 (1)	10.00			
R- Q	0 / 7461	-28.0	-28.0	0.43 (1)	10.00			
Q- P	0 / 7461	-28.0	-28.0	0.43 (1)	10.00			
P- O	0 / 5942	-28.0	-28.0	0.35 (1)	10.00			
O- N	0 / 5942	-28.0	-28.0	0.35 (1)	10.00			
N- M	0 / 4097	-28.0	-28.0	0.26 (1)	10.00			
M- V	0 / 3896	-28.0	-28.0	0.26 (1)	10.00			
V- K	0 / 1979	-28.0	-28.0	0.12 (2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
D	8-3-10	-45	-50	—	FRONT	VERT	DEAD
D	8-3-10	-455	-455	—	FRONT	VERT	SNOW
S	8-0-8	-2507	-2507	—	BACK	VERT	TOTAL
X	2-1-4	-41	-72	—	BACK	VERT	TOTAL
Y	4-1-4	-41	-72	—	BACK	VERT	TOTAL
Z	6-1-4	-41	-72	—	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP	CH.	LL =	25.6	PSF
		DL =	3.0	PSF
BOT	CH.	LL =	10.5	PSF
		DL =	7.0	PSF
TOTAL	LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA O86-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.29")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.29")
ALLOWABLE DEFL.(TL)= L/360 (1.29")
CALCULATED VERT. DEFL.(TL) = L/ 994 (0.47")

CSI: TC=0.48 (C-D:1) , BC=0.45 (S-T:1) , WB=0.58 (C-T:1) , SSI=0.27 (B-T:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (N) (INPUT = 0.90)
JSI METAL= 1.00 (J) (INPUT = 1.00)

STRUCTURAL 41950
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285473	T115	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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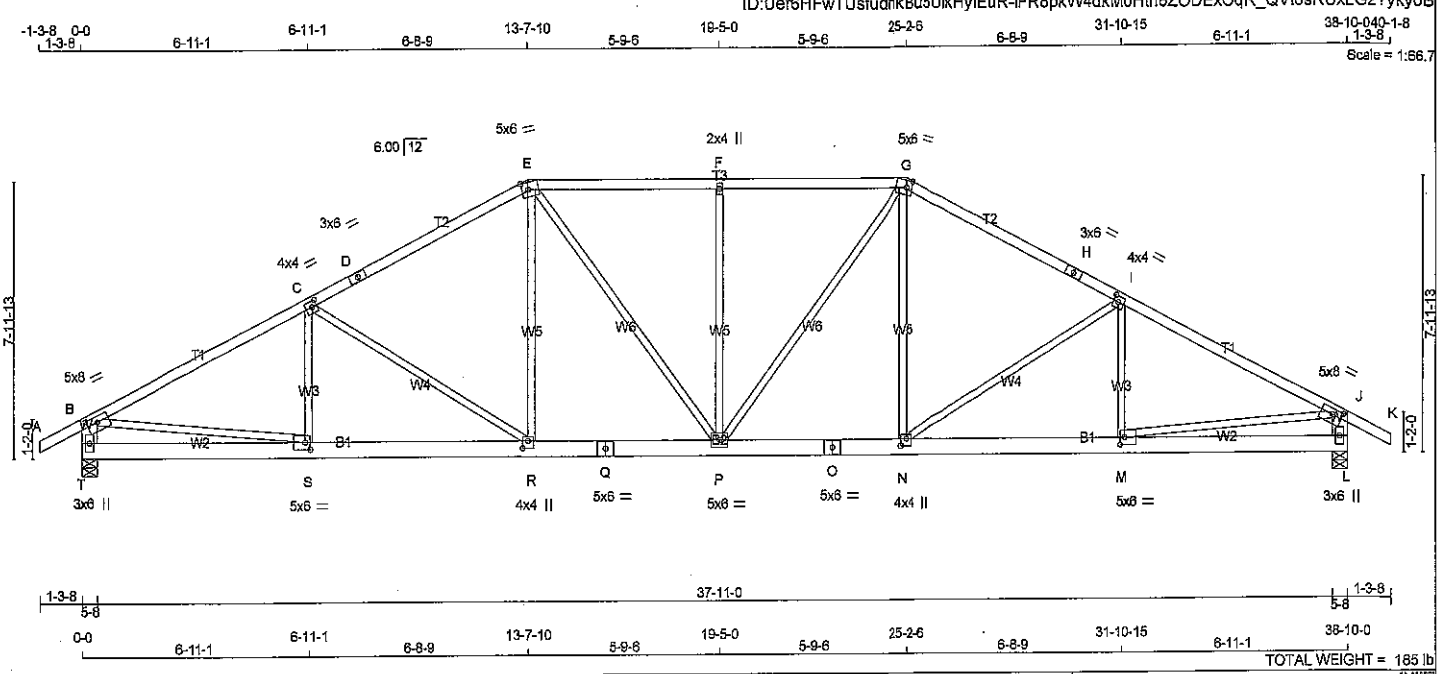


[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
T - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
T - Q	2x6	DRY	No.2	SPF
Q - O	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.00	3.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	5.0	6.0	2.50	2.25
F	TMW+w	MT20	2.0	4.0		
G	TTWW-m	MT20	5.0	6.0	2.50	2.25
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	4.0	4.0	2.00	1.75
J	TMVW-t	MT20	5.0	8.0	2.00	3.75
L	BMV1+p	MT20	3.0	6.0		
M	BMWW-t	MT20	5.0	6.0	2.50	1.75
N	BMWW+t	MT20	4.0	4.0	2.50	2.00
O	BS-t	MT20	5.0	6.0		
P	BMWWW-t	MT20	5.0	6.0		
Q	BS-t	MT20	5.0	6.0		
R	BMWW+t	MT20	4.0	4.0	2.50	2.00
S	BMWW-t	MT20	5.0	6.0	2.50	1.75
T	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	2294	0	2294	0	0	5-8	5-8
L	2294	0	2294	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1867	1063 / 0	408 / 0	0 / 0	0 / 0	396 / 0	0 / 0
L	1867	1063 / 0	408 / 0	0 / 0	0 / 0	396 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

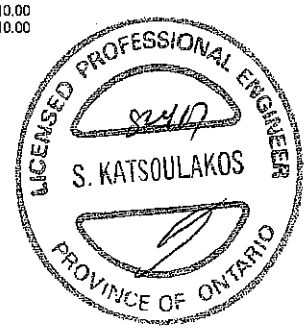
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.10 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A-B	0 / 26	-84.3	-84.3	0.11 (1)	10.00	S-C	-156 / 199	0.05 (1)
B-C	-3372 / 0	-84.3	-84.3	0.83 (1)	3.10	C-R	-586 / 0	0.78 (1)
C-D	-2878 / 0	-84.3	-84.3	0.72 (1)	3.43	R-E	0 / 533	0.12 (2)
D-E	-2878 / 0	-84.3	-84.3	0.72 (1)	3.43	E-P	0 / 456	0.10 (1)
E-F	-2828 / 0	-84.3	-84.3	0.48 (1)	3.70	P-F	-595 / 0	0.68 (1)
F-G	-2828 / 0	-84.3	-84.3	0.48 (1)	3.70	P-G	0 / 456	0.10 (1)
G-H	-2878 / 0	-84.3	-84.3	0.72 (1)	3.43	N-G	0 / 533	0.12 (2)
H-I	-2878 / 0	-84.3	-84.3	0.72 (1)	3.43	N-I	-586 / 0	0.78 (1)
I-J	-3372 / 0	-84.3	-84.3	0.83 (1)	3.10	M-I	-156 / 199	0.05 (1)
J-K	0 / 26	-84.3	-84.3	0.11 (1)	10.00	B-S	0 / 3062	0.69 (1)
T-B	-2201 / 0	0.0	0.0	0.14 (1)	6.88	M-J	0 / 3062	0.69 (1)
L-J	-2201 / 0	0.0	0.0	0.14 (1)	6.88			
T-S	0 / 0	-28.0	-28.0	0.14 (2)	10.00			
S-R	0 / 3043	-28.0	-28.0	0.43 (1)	10.00			
R-Q	0 / 2552	-28.0	-28.0	0.37 (1)	10.00			
Q-P	0 / 2552	-28.0	-28.0	0.37 (1)	10.00			
P-O	0 / 2552	-28.0	-28.0	0.37 (1)	10.00			
O-N	0 / 2552	-28.0	-28.0	0.37 (1)	10.00			
N-M	0 / 3043	-28.0	-28.0	0.43 (1)	10.00			
M-L	0 / 0	-28.0	-28.0	0.14 (2)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.29")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.21")
ALLOWABLE DEFL.(TL)= L/360 (1.29")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.33")

CSI: TC=0.83 (B-C:1) , BC=0.43 (R-S:1) , WB=0.78 (C-R:1) , SSI=0.25 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (R) (INPUT = 0.90)
JSI METAL= 0.76 (S) (INPUT = 1.00)

DWG NO. TAM41952-17
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285473	T116	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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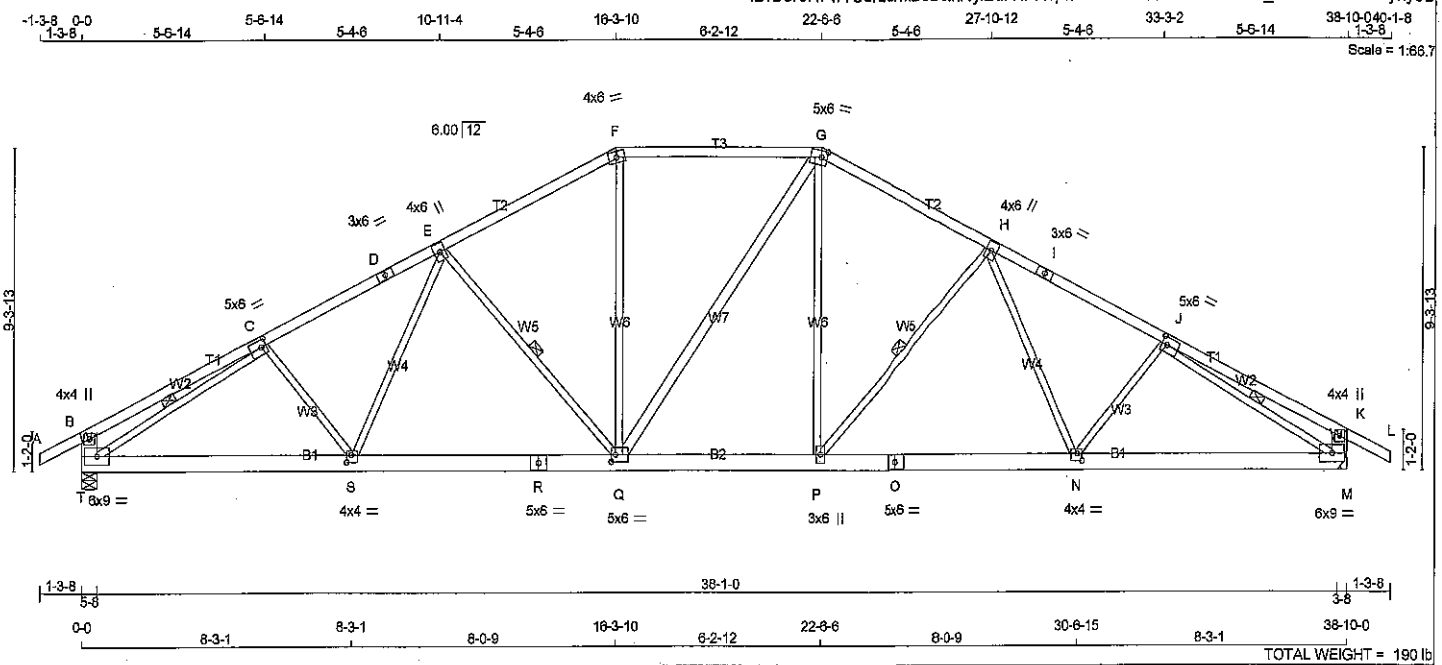

TOTAL WEIGHT = 190 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
T - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
T - R	2x6	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
O - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
Q - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	1.75
D	TS-t	MT20	3.0	6.0		
E	TMWW+t	MT20	4.0	6.0		
F	TTW-m	MT20	4.0	6.0		
G	TTWW-m	MT20	5.0	6.0	2.25	2.00
H	TMWW+t	MT20	4.0	6.0		
I	TS-t	MT20	3.0	6.0		
J	TMWW-t	MT20	5.0	6.0	2.50	1.75
K	TMV+p	MT20	4.0	4.0		
M	BMVW1-t	MT20	6.0	9.0		
N	BMWW-t	MT20	4.0	4.0	2.50	1.75
O	BS-t	MT20	5.0	6.0		
P	BMWW+t	MT20	3.0	6.0		
Q	BMWWW-t	MT20	5.0	6.0	2.50	1.50
R	BS-t	MT20	5.0	6.0		
S	BMWW-t	MT20	4.0	4.0	2.50	1.75
T	BMVW1-t	MT20	6.0	9.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
T	2294	0	2294	0	0	5-8	5-8
M	2294	0	2294	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1867	1063 / 0	408 / 0	0 / 0	0 / 0	396 / 0	0 / 0
M	1867	1063 / 0	408 / 0	0 / 0	0 / 0	396 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.57 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-Q, H-P, C-T, J-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF)	CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 26	-84.3	-84.3	0.11 (1)	10.00	
B- C	0 / 16	-84.3	-84.3	0.30 (1)	10.00	
C- D	-3236 / 0	-84.3	-84.3	0.45 (1)	3.58	
D- E	-3236 / 0	-84.3	-84.3	0.45 (1)	3.58	
E- F	-2608 / 0	-84.3	-84.3	0.40 (1)	3.96	
F- G	-2322 / 0	-84.3	-84.3	0.52 (1)	3.97	
G- H	-2606 / 0	-84.3	-84.3	0.40 (1)	3.96	
H- I	-3237 / 0	-84.3	-84.3	0.45 (1)	3.57	
I- J	-3237 / 0	-84.3	-84.3	0.45 (1)	3.57	
J- K	0 / 18	-84.3	-84.3	0.30 (1)	10.00	
K- L	0 / 26	-84.3	-84.3	0.11 (1)	10.00	
T- B	-305 / 0	0.0	0.0	0.02 (1)	7.81	
M- K	-305 / 0	0.0	0.0	0.02 (1)	7.81	
T- S	0 / 2842	-28.0	-28.0	0.47 (1)	10.00	
S- R	0 / 2767	-28.0	-28.0	0.43 (2)	10.00	
R- Q	0 / 2767	-28.0	-28.0	0.43 (2)	10.00	
Q- P	0 / 2320	-28.0	-28.0	0.35 (1)	10.00	
P- O	0 / 2767	-28.0	-28.0	0.44 (2)	10.00	
O- N	0 / 2767	-28.0	-28.0	0.44 (2)	10.00	
N- M	0 / 2943	-28.0	-28.0	0.47 (1)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- S	-74 / 148	0.03 (3)
S- E	0 / 406	0.09 (2)
E- Q	-689 / 0	0.32 (1)
Q- F	0 / 717	0.16 (1)
Q- G	0 / 4	0.00 (2)
P- G	0 / 713	0.16 (1)
P- H	-691 / 0	0.32 (1)
H- N	0 / 410	0.09 (2)
N- J	-74 / 148	0.03 (3)
T- C	-3496 / 0	0.96 (1)
J- M	-3497 / 0	0.96 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.29")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.20")
ALLOWABLE DEFL.(TL)= L/360 (1.29")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.32")

CSI: TC=0.52 (F-G:1) , BC=0.47 (S-T:1) , WB=0.96 (J-M:1) , SSI=0.20 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (J) (INPUT = 0.90)
JSI METAL= 0.88 (J) (INPUT = 1.00)

DWG NO. TAM41953-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285473	T117	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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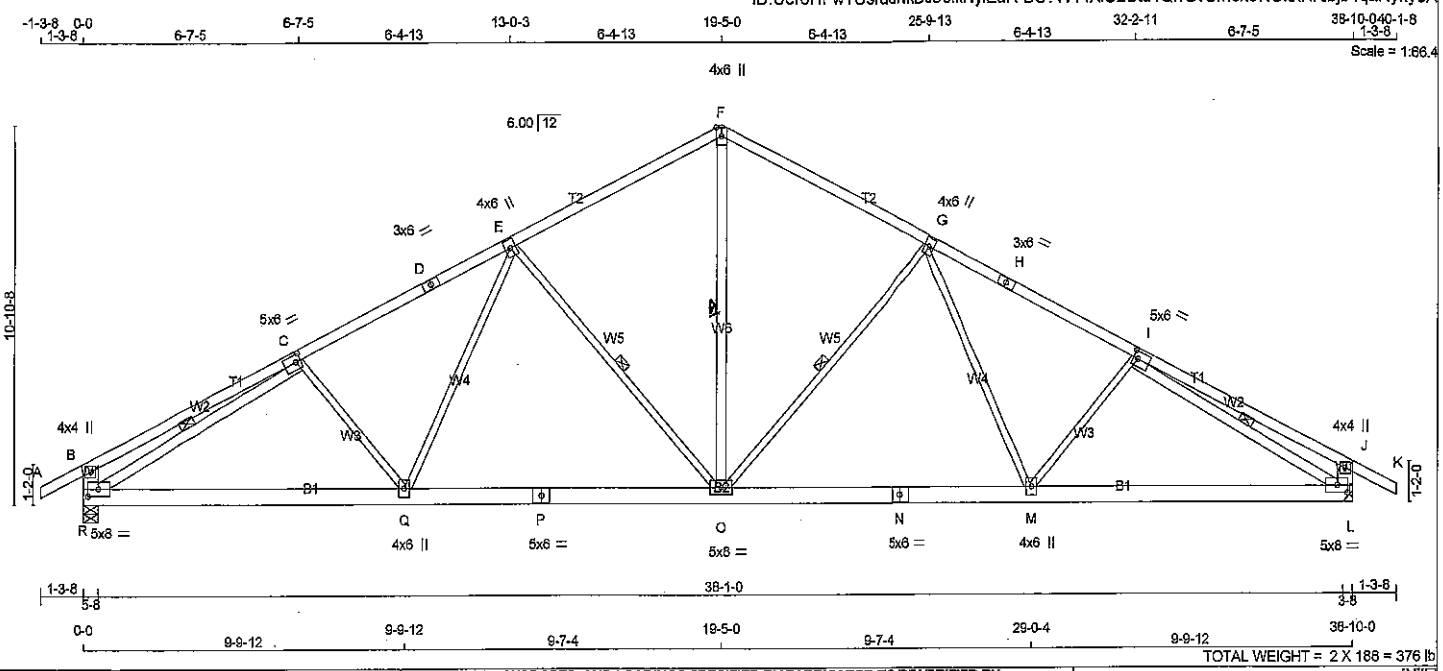

TOTAL WEIGHT = 2 X 188 = 376 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
R - P	2x6	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
O - F	2x4	DRY	No.2	SPF
R - C	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	1.75
D	TS-t	MT20	3.0	6.0		
E	TMWW+t	MT20	4.0	6.0		
F	TTW+p	MT20	4.0	6.0	Edge	
G	TMWW+t	MT20	4.0	6.0		
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	5.0	6.0	2.50	1.75
J	TMV+p	MT20	4.0	4.0		
L	BMVW1-t	MT20	5.0	8.0	2.50	3.75
M	BMWW+t	MT20	4.0	6.0		
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	8.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWW+t	MT20	4.0	6.0		
R	BMVW1-t	MT20	5.0	8.0	2.50	3.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
R	2294	0	2294	0	0	IN-SX 5-8	IN-SX 5-8
L	2294	0	2294	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1867	1063 / 0	408 / 0	0 / 0	0 / 0	396 / 0	0 / 0
L	1867	1063 / 0	408 / 0	0 / 0	0 / 0	396 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.47 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-O, E-O, C-R, I-L. F-O

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 26	-84.3	-84.3	0.11 (1)	10.00
B-C	0 / 23	-84.3	-84.3	0.46 (1)	10.00
C-D	-3164 / 0	-84.3	-84.3	0.63 (1)	3.47
D-E	-3164 / 0	-84.3	-84.3	0.63 (1)	3.47
E-F	-2302 / 0	-84.3	-84.3	0.54 (1)	4.01
F-G	-2302 / 0	-84.3	-84.3	0.54 (1)	4.01
G-H	-3164 / 0	-84.3	-84.3	0.63 (1)	3.47
H-I	-3164 / 0	-84.3	-84.3	0.63 (1)	3.47
I-J	0 / 23	-84.3	-84.3	0.46 (1)	10.00
J-K	0 / 26	-84.3	-84.3	0.11 (1)	10.00
R-B	-337 / 0	0.0	0.0	0.02 (1)	7.81
L-J	-337 / 0	0.0	0.0	0.02 (1)	7.81
R-Q	0 / 2973	-28.0	-28.0	0.54 (2)	10.00
Q-P	0 / 2626	-28.0	-28.0	0.52 (2)	10.00
P-O	0 / 2626	-28.0	-28.0	0.52 (2)	10.00
O-N	0 / 2626	-28.0	-28.0	0.52 (2)	10.00
N-M	0 / 2626	-28.0	-28.0	0.52 (2)	10.00
M-L	0 / 2973	-28.0	-28.0	0.54 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O-F	0 / 1605	0.28 (1)
O-G	-891 / 0	0.59 (1)
G-M	0 / 566	0.13 (2)
M-I	-224 / 109	0.10 (1)
E-O	-891 / 0	0.59 (1)
Q-E	0 / 566	0.13 (2)
C-Q	-224 / 109	0.10 (1)
R-C	-3497 / 0	0.93 (1)
I-L	-3497 / 0	0.93 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.29")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.20")
ALLOWABLE DEFL.(TL)= L/360 (1.29")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.32")

CSI: TC=0.63 (G-I:1) , BC=0.54 (Q-R:2) , WB=0.93 (C-R:1) , SSI=0.23 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (R) (INPUT = 0.90)
JSI METAL= 0.86 (C) (INPUT = 1.00)

DWG NO. TAM 41954-17
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285473	T117C	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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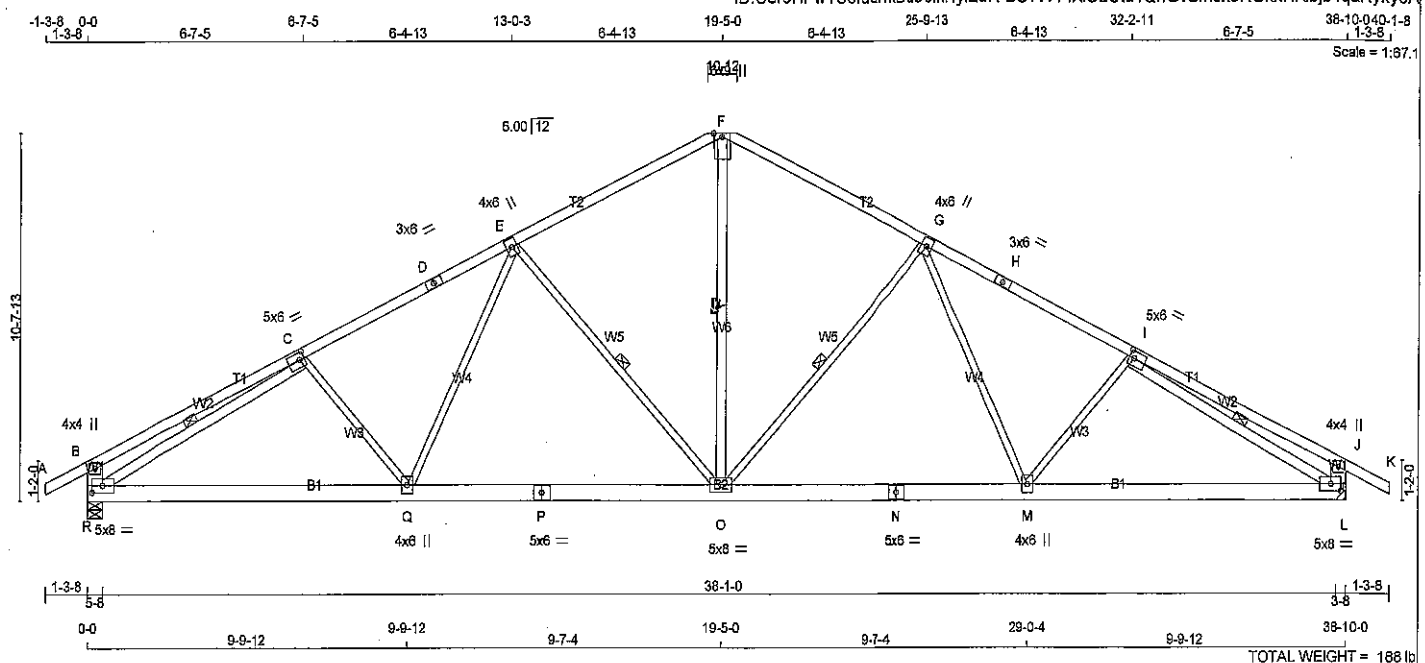


LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
R - P	2x6	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
O - F	2x4	DRY	No.2	SPF
R - C	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	1.75
D	TS-t	MT20	3.0	6.0		
E	TMWW+t	MT20	4.0	6.0		
F	TMTMW+p	MT20	6.0	9.0	Edge	
G	TMWW+t	MT20	4.0	6.0		
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	5.0	6.0	2.50	1.75
J	TMV+p	MT20	4.0	4.0		
L	BMWW1-t	MT20	5.0	6.0	2.50	3.75
M	BMWW+t	MT20	4.0	6.0		
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	6.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWW+t	MT20	4.0	6.0		
R	BMWW1-t	MT20	5.0	6.0	2.50	3.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2294	0	2294	0	0	5-8	5-8
L	2294	0	2294	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1867	1063 / 0	408 / 0	0 / 0	0 / 0	396 / 0	0 / 0
L	1867	1063 / 0	408 / 0	0 / 0	0 / 0	396 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.47 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

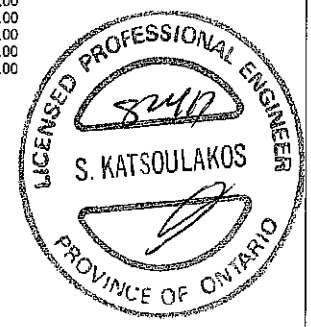
1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-O, E-O, C-R, I-L. F-O

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 26	-84.3	-84.3	0.11 (1)	10.00
B-C	0 / 23	-84.3	-84.3	0.46 (1)	10.00
C-D	-3164 / 0	-84.3	-84.3	0.63 (1)	3.47
D-E	-3164 / 0	-84.3	-84.3	0.63 (1)	3.47
E-F	-2302 / 0	-84.3	-84.3	0.54 (1)	4.01
F-G	-2302 / 0	-84.3	-84.3	0.54 (1)	4.01
G-H	-3164 / 0	-84.3	-84.3	0.63 (1)	3.47
H-I	-3164 / 0	-84.3	-84.3	0.63 (1)	3.47
I-J	0 / 23	-84.3	-84.3	0.46 (1)	10.00
J-K	0 / 26	-84.3	-84.3	0.11 (1)	10.00
R-B	-337 / 0	0.0	0.0	0.02 (1)	7.81
L-J	-337 / 0	0.0	0.0	0.02 (1)	7.81
R-Q	0 / 2973	-28.0	-28.0	0.54 (2)	10.00
Q-P	0 / 2626	-28.0	-28.0	0.52 (2)	10.00
P-O	0 / 2626	-28.0	-28.0	0.52 (2)	10.00
O-N	0 / 2626	-28.0	-28.0	0.52 (2)	10.00
N-M	0 / 2626	-28.0	-28.0	0.52 (2)	10.00
M-L	0 / 2973	-28.0	-28.0	0.54 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O-F	0 / 1605	0.26 (1)
O-G	-891 / 0	0.59 (1)
G-M	0 / 566	0.13 (2)
M-I	-224 / 109	0.10 (1)
E-O	-891 / 0	0.59 (1)
Q-E	0 / 566	0.13 (2)
C-Q	-224 / 109	0.10 (1)
R-C	-3497 / 0	0.93 (1)
I-L	-3497 / 0	0.93 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.28")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.20")
ALLOWABLE DEFL.(TL)= L/360 (1.28")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.32")

CSI: TC=0.63 (C-E:1) , BC=0.54 (Q-R:2) , WB=0.93 (C-R:1) , SSI=0.23 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (R) (INPUT = 0.90)
JSI METAL= 0.86 (I) (INPUT = 1.00)

DWG NO. TAM 41955-17
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285473	T118A	1	1	TRUSS DESC.	

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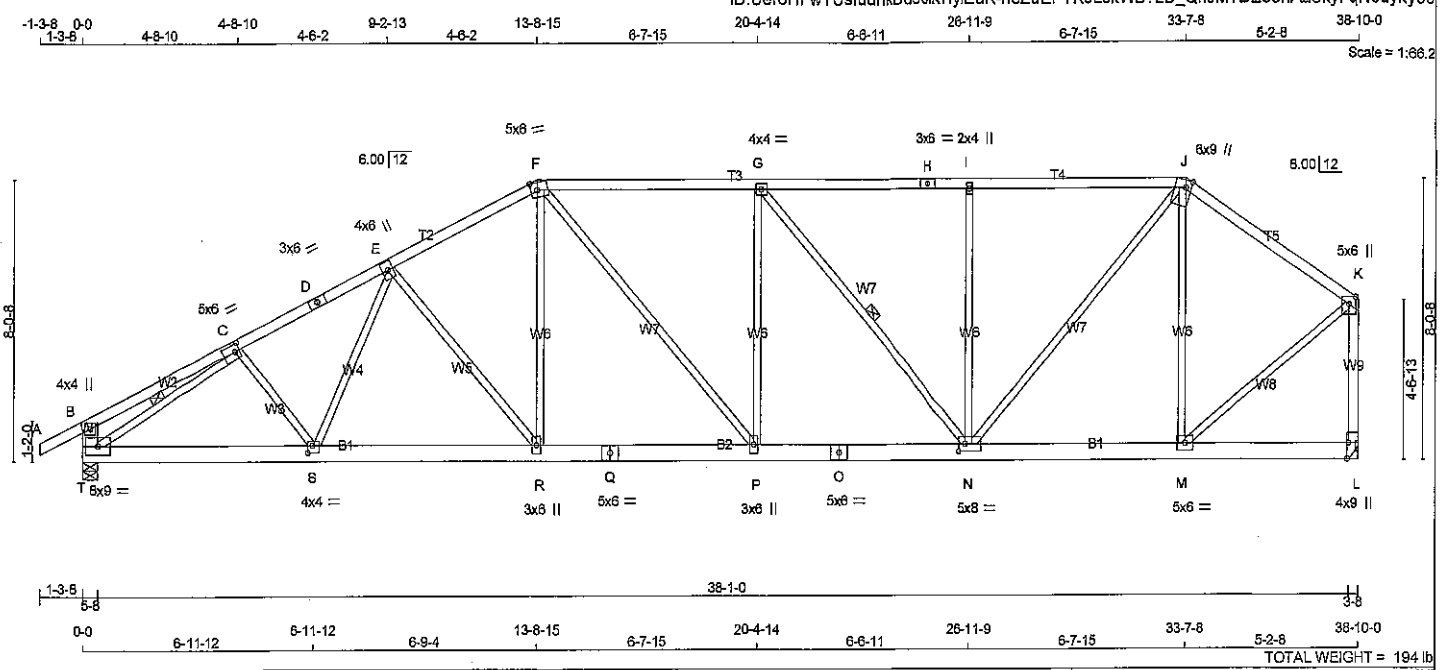


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
J - K	2x4	DRY	No.2	SPF
T - B	2x6	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
T - Q	2x6	DRY	No.2	SPF
Q - O	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	1.75
D	TS-t	MT20	3.0	6.0		
E	TMWW+t	MT20	4.0	6.0		
F	TTWW-m	MT20	5.0	6.0	2.50	2.25
G	TMWW-t	MT20	4.0	4.0		
H	TS-t	MT20	3.0	6.0		
I	TMW+w	MT20	2.0	4.0		
J	TTWW+m	MT20	6.0	9.0	Edge	2.00
K	TMVW+p	MT20	5.0	6.0	Edge	
L	BMV1+t	MT20	4.0	9.0	Edge	0.50
M	BMWW-t	MT20	5.0	6.0		
N	BMWWW-t	MT20	5.0	8.0	2.50	2.50
O	BS-t	MT20	5.0	6.0		
P	BMWW+t	MT20	3.0	6.0		
Q	BS-t	MT20	5.0	6.0		
R	BMWW+t	MT20	3.0	6.0		
S	BMWW-t	MT20	4.0	4.0	2.50	1.75
T	BMVW1-t	MT20	6.0	9.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
T	2294	0	2294	0	0	IN-SX	IN-SX
L	2180	0	2180	0	0	5-8	5-8
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1867	1063 / 0	408 / 0	0 / 0	0 / 0	396 / 0	0 / 0
L	1790	994 / 0	408 / 0	0 / 0	0 / 0	386 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.38 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-N, C-T.

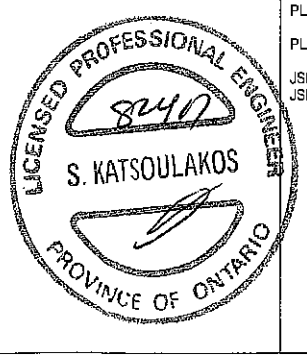
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 26	-84.3	-84.3	0.11 (1)	10.00
B- C	0 / 14	-84.3	-84.3	0.22 (1)	10.00
C- D	-3262 / 0	-84.3	-84.3	0.43 (1)	3.55
D- E	-3262 / 0	-84.3	-84.3	0.43 (1)	3.55
E- F	-2853 / 0	-84.3	-84.3	0.38 (1)	3.80
F- G	-2810 / 0	-84.3	-84.3	0.81 (1)	3.38
G- H	-2396 / 0	-84.3	-84.3	0.73 (1)	3.68
H- I	-2396 / 0	-84.3	-84.3	0.73 (1)	3.68
I- J	-2396 / 0	-84.3	-84.3	0.73 (1)	3.70
J- K	-1580 / 0	-84.3	-84.3	0.49 (1)	4.69
T- B	-278 / 0	0.0	0.0	0.02 (1)	7.81
L- K	-2127 / 0	0.0	0.0	0.70 (1)	5.81
T- S	0 / 2882	-28.0	-28.0	0.44 (1)	10.00
S- R	0 / 2861	-28.0	-28.0	0.41 (1)	10.00
R- Q	0 / 2543	-28.0	-28.0	0.37 (1)	10.00
Q- P	0 / 2543	-28.0	-28.0	0.37 (1)	10.00
P- O	0 / 2811	-28.0	-28.0	0.39 (1)	10.00
O- N	0 / 2811	-28.0	-28.0	0.39 (1)	10.00
N- M	0 / 1302	-28.0	-28.0	0.24 (2)	10.00
M- L	0 / 0	-28.0	-28.0	0.12 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- S	0 / 186	0.04 (3)
S- E	0 / 247	0.06 (2)
E- R	-501 / 0	0.48 (1)
R- F	0 / 582	0.13 (1)
F- P	0 / 407	0.09 (1)
P- G	-124 / 190	0.14 (1)
G- N	-637 / 0	0.46 (1)
N- I	-604 / 0	0.70 (1)
N- J	0 / 1667	0.38 (1)
M- J	-830 / 0	0.97 (1)
T- C	-3485 / 0	0.76 (1)
M- K	0 / 1668	0.38 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.29")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (1.29")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.28")

CSI: TC=0.81 (F-G:1) , BC=0.44 (S-T:1) , WB=0.97 (J-M:1) , SSI=0.26 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

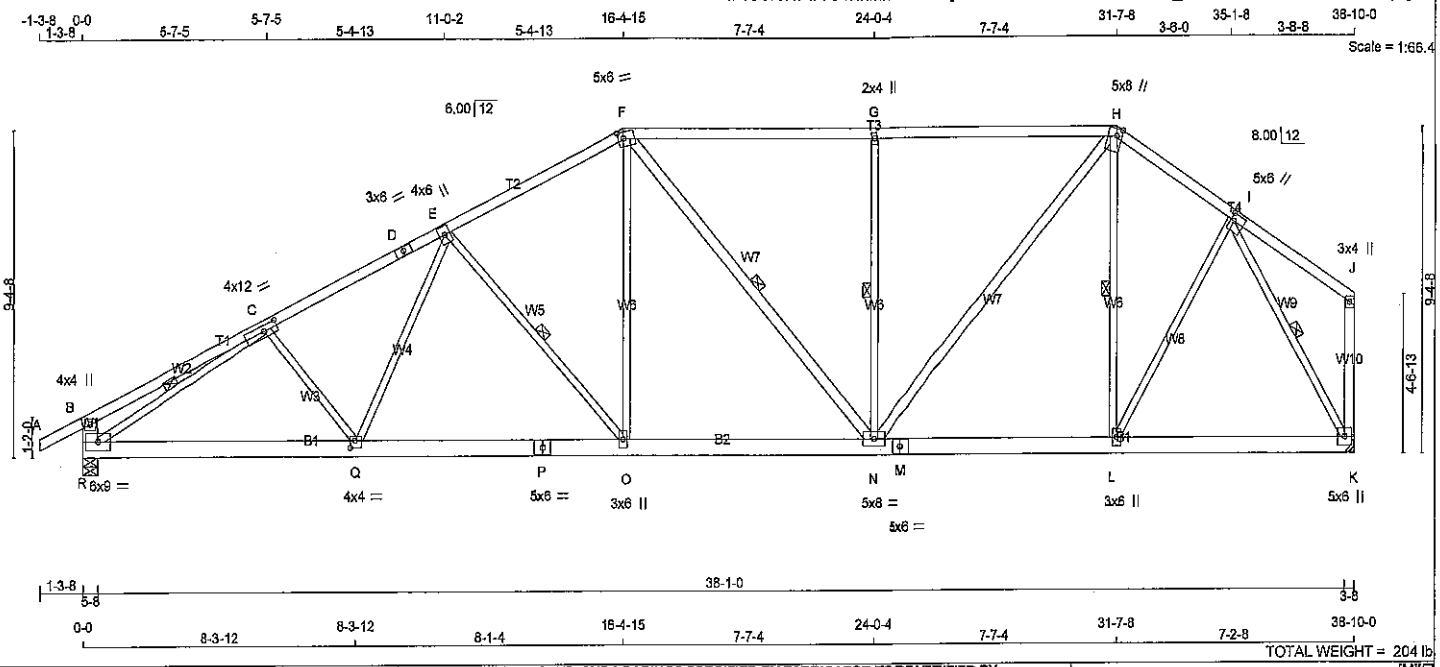
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.86 (C) (INPUT = 1.00)

DWG NO. TAM 41956-17
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285473	T119A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington



TOTAL WEIGHT = 204 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
R - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
F - N	2x4	DRY	No.2	SPF
N - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	4.0	12.0	2.00	5.00
D	TS-t	MT20	3.0	6.0		
E	TMWW+t	MT20	4.0	6.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMW+w	MT20	2.0	4.0		
H	TTWW+m	MT20	5.0	6.0	Edge	1.75
I	TMWW+t	MT20	5.0	6.0	2.50	1.25
J	TMV+p	MT20	3.0	4.0		
K	BMVW1+p	MT20	5.0	6.0		
L	BMWW+t	MT20	3.0	6.0		
M	BS-t	MT20	5.0	6.0		
N	BMWWW-t	MT20	5.0	8.0		
O	BMWW+t	MT20	3.0	6.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWW-t	MT20	4.0	4.0	2.50	1.75
R	BMVW1-t	MT20	6.0	9.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT	REQRD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG IN-SX	BRG IN-SX
R	2294	0	2294	0	0	5-8	5-8
K	2180	0	2180	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1867	1063 / 0	408 / 0	0 / 0	0 / 0	396 / 0	0 / 0
K	1790	994 / 0	408 / 0	0 / 0	0 / 0	388 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.56 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

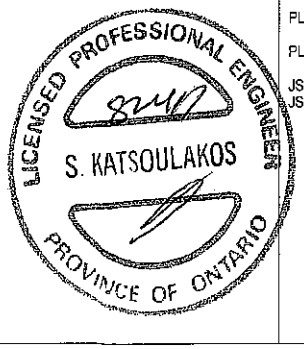
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-O, F-N, G-N, H-L, C-R, I-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-84.3	-84.3	0.11 (1)	10.00
B- C	0 / 18	-84.3	-84.3	0.31 (1)	10.00
C- D	-3233 / 0	-84.3	-84.3	0.45 (1)	3.57
D- E	-3233 / 0	-84.3	-84.3	0.45 (1)	3.57
E- F	-2600 / 0	-84.3	-84.3	0.40 (1)	3.96
F- G	-2296 / 0	-84.3	-84.3	0.78 (1)	3.56
G- H	-2296 / 0	-84.3	-84.3	0.78 (1)	3.56
H- I	-1762 / 0	-84.3	-84.3	0.17 (1)	4.89
I- J	0 / 19	-84.3	-84.3	0.17 (1)	10.00
R- B	-306 / 0	0.0	0.0	0.02 (1)	7.81
K- J	-116 / 0	0.0	0.0	0.04 (1)	7.81
R- Q	0 / 2943	-28.0	-28.0	0.47 (1)	10.00
Q- P	0 / 2763	-28.0	-28.0	0.43 (2)	10.00
P- O	0 / 2763	-28.0	-28.0	0.43 (2)	10.00
O- N	0 / 2315	-28.0	-28.0	0.37 (1)	10.00
N- M	0 / 1443	-28.0	-28.0	0.31 (2)	10.00
M- L	0 / 1443	-28.0	-28.0	0.31 (2)	10.00
L- K	0 / 1083	-28.0	-28.0	0.27 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- Q	-80 / 146	0.03 (3)
Q- E	0 / 406	0.09 (2)
E- O	-694 / 0	0.33 (1)
O- F	0 / 752	0.17 (1)
F- N	-60 / 0	0.05 (3)
N- G	-791 / 0	0.44 (1)
N- H	0 / 1320	0.21 (1)
L- H	-402 / 0	0.22 (1)
L- I	0 / 736	0.17 (1)
R- C	-3495 / 0	0.97 (1)
I- K	-2259 / 0	0.82 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.29")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (1.29")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.28")

CSI: TC=0.78 (F-G:1) , BC=0.47 (Q-R:1) , WB=0.97 (C-R:1) , SSI=0.31 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1556

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (C) (INPUT = 0.90)
JSI METAL= 0.57 (I) (INPUT = 1.00)

DWG NO. TAM 41957-17
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285473	T120A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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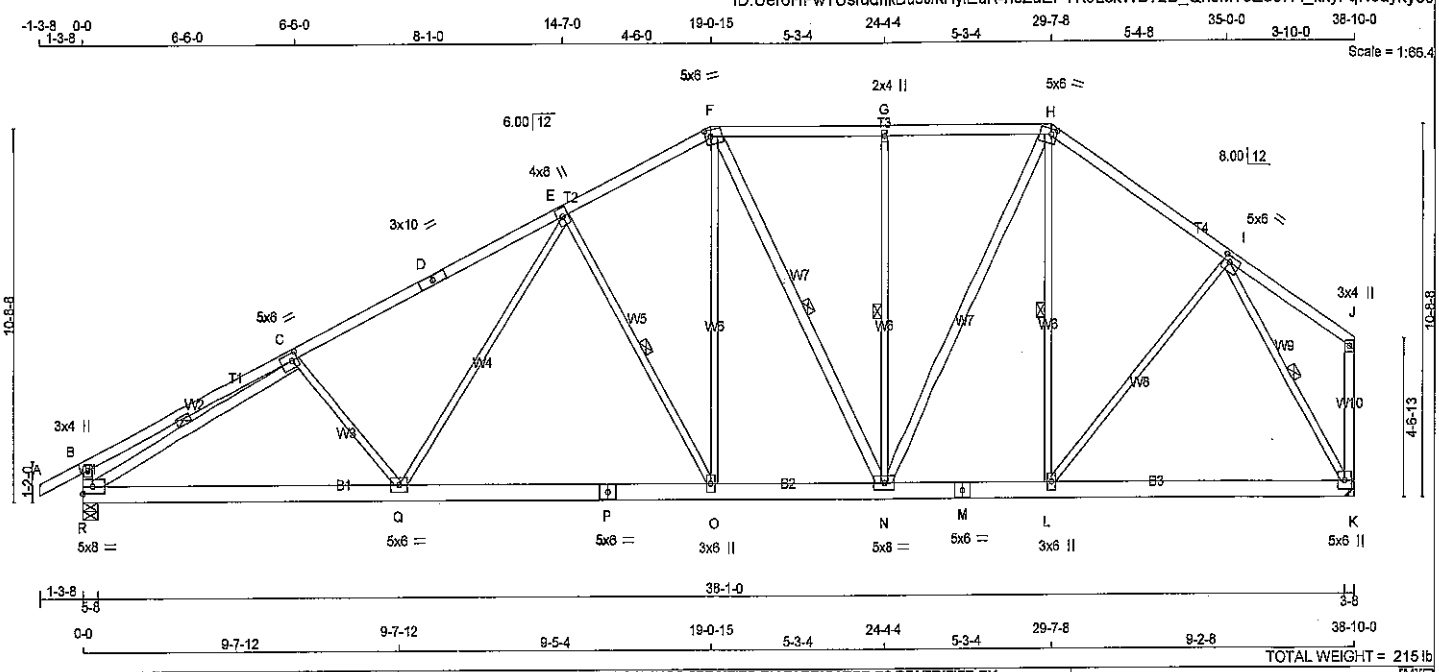

TOTAL WEIGHT = 215 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
R - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
F - N	2x4	DRY	No.2	SPF
N - H	2x4	DRY	No.2	SPF
R - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.00
D	TS-t	MT20	3.0	10.0		
E	TMWW+t	MT20	4.0	6.0		
F	TTWW-m	MT20	5.0	8.0	2.25	2.00
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	6.0	1.75	2.00
I	TMWW-t	MT20	5.0	6.0	2.00	2.25
J	TMV+p	MT20	3.0	4.0		
K	BMVW1+p	MT20	5.0	6.0		
L	BMWW+t	MT20	3.0	6.0		
M	BS-t	MT20	5.0	6.0		
N	BMWWW-t	MT20	5.0	6.0		
O	BMWW+t	MT20	3.0	6.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWW-t	MT20	5.0	6.0		
R	BMVW1-t	MT20	5.0	8.0		Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2294	0	2294	0	0	5-8	5-8
K	2160	0	2160	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1867	1063 / 0	408 / 0	0 / 0	0 / 0	396 / 0	0 / 0
K	1790	994 / 0	408 / 0	0 / 0	0 / 0	388 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.23 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-O, F-N, G-N, H-L, C-R, I-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 26	-84.3	-84.3	0.11 (1)	10.00	C- Q	-303 / 105	0.13 (1)
B- C	0 / 32	-84.3	-84.3	0.62 (1)	10.00	Q- E	0 / 639	0.14 (2)
C- D	-3153 / 0	-84.3	-84.3	0.99 (1)	3.23	E- O	-877 / 0	0.52 (1)
D- E	-3153 / 0	-84.3	-84.3	0.99 (1)	3.23	O- F	0 / 990	0.22 (1)
E- F	-2335 / 0	-84.3	-84.3	0.47 (1)	4.00	F- N	-269 / 0	0.20 (1)
F- G	-1942 / 0	-84.3	-84.3	0.35 (1)	4.49	N- G	-542 / 0	0.42 (1)
G- H	-1942 / 0	-84.3	-84.3	0.35 (1)	4.49	N- H	0 / 958	0.15 (1)
H- I	-1839 / 0	-84.3	-84.3	0.36 (1)	4.61	L- H	-208 / 135	0.16 (1)
I- J	0 / 35	-84.3	-84.3	0.32 (1)	10.00	L- I	0 / 622	0.14 (1)
R- B	-312 / 0	0.0	0.0	0.03 (1)	7.81	R- C	-3541 / 0	0.95 (1)
K- J	-93 / 0	0.0	0.0	0.03 (1)	7.81	I- K	-2268 / 0	0.85 (1)
R- Q	0 / 3008	-28.0	-28.0	0.55 (2)	10.00			
Q- P	0 / 2490	-28.0	-28.0	0.51 (2)	10.00			
P- O	0 / 2490	-28.0	-28.0	0.51 (2)	10.00			
O- N	0 / 2064	-28.0	-28.0	0.35 (2)	10.00			
N- M	0 / 1508	-28.0	-28.0	0.39 (2)	10.00			
M- L	0 / 1506	-28.0	-28.0	0.39 (2)	10.00			
L- K	0 / 1107	-28.0	-28.0	0.35 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.29")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (1.29")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.30")

CSI: TC=0.99 (C-E:1) , BC=0.55 (Q-R:2) , WB=0.95 (C-R:1) , SSI=0.25 (C-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.86 (C) (INPUT = 1.00)

DWG NO. TAM 41958.17

STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285473	T121X	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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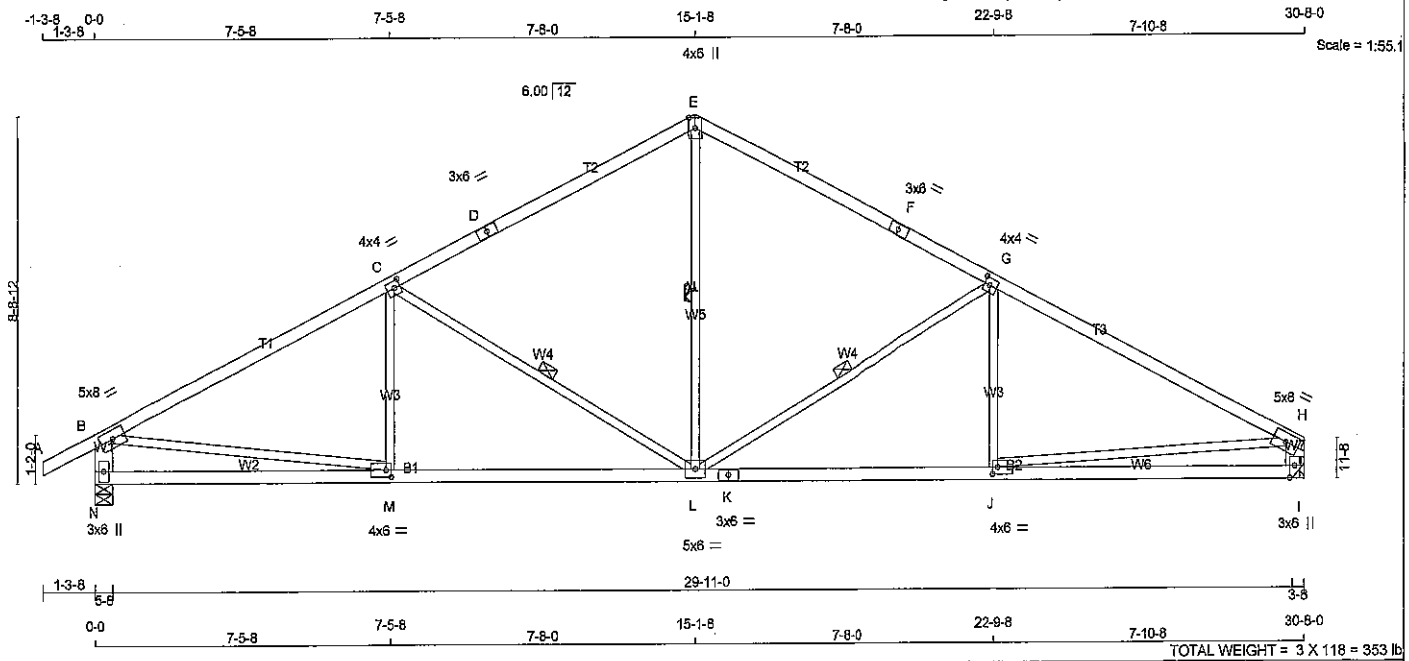

TOTAL WEIGHT = 3 X 118 = 353 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
N - B	2x6	DRY	No.2	SPF
I - H	2x6	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	6.0		
E	TTW+p	MT20	4.0	6.0	Edge	
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
H	TMVW-t	MT20	5.0	8.0		
I	BMV1+p	MT20	3.0	6.0	Edge	1.50
J	BMWW-t	MT20	4.0	6.0	2.00	1.50
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	5.0	6.0		
M	BMWW-t	MT20	4.0	6.0	2.00	1.50
N	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1836	0	1836	0	0	5-8	5-8
I	1721	0	1721	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1491	854 / 0	322 / 0	0 / 0	0 / 0	315 / 0	0 / 0
I	1413	785 / 0	322 / 0	0 / 0	0 / 0	307 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-L, G-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 26	-84.3	-84.3	0.11 (1)	10.00	
B- C	-2510 / 0	-84.3	-84.3	0.85 (1)	3.42	
C- D	-1803 / 0	-84.3	-84.3	0.76 (1)	4.01	
D- E	-1803 / 0	-84.3	-84.3	0.76 (1)	4.01	
E- F	-1804 / 0	-84.3	-84.3	0.79 (1)	3.94	
F- G	-1804 / 0	-84.3	-84.3	0.79 (1)	3.94	
G- H	-2608 / 0	-84.3	-84.3	0.98 (1)	3.20	
N- B	-1750 / 0	0.0	0.0	0.11 (1)	7.51	
I- H	-1631 / 0	0.0	0.0	0.10 (1)	7.71	
N- M	0 / 0	-28.0	-28.0	0.39 (3)	10.00	
M- L	0 / 2276	-28.0	-28.0	0.65 (2)	10.00	
L- K	0 / 2364	-28.0	-28.0	0.68 (2)	10.00	
K- J	0 / 2364	-28.0	-28.0	0.68 (2)	10.00	
J- I	0 / 0	-28.0	-28.0	0.41 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- C	-28 / 288	0.06 (3)
C- L	-809 / 0	0.45 (1)
L- E	0 / 1092	0.25 (1)
L- G	-911 / 0	0.50 (1)
J- G	0 / 334	0.08 (3)
B- M	0 / 2291	0.52 (1)
J- H	0 / 2372	0.53 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.02")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (1.02")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.30")

CSI: TC=0.98 (G-H:1) , BC=0.68 (J-L:2) , WB=0.53 (H-J:1) , SSI=0.29 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (M) (INPUT = 0.90)
JSI METAL= 0.70 (K) (INPUT = 1.00)

DWG NO. TAM 41959-17

STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
286047	T122S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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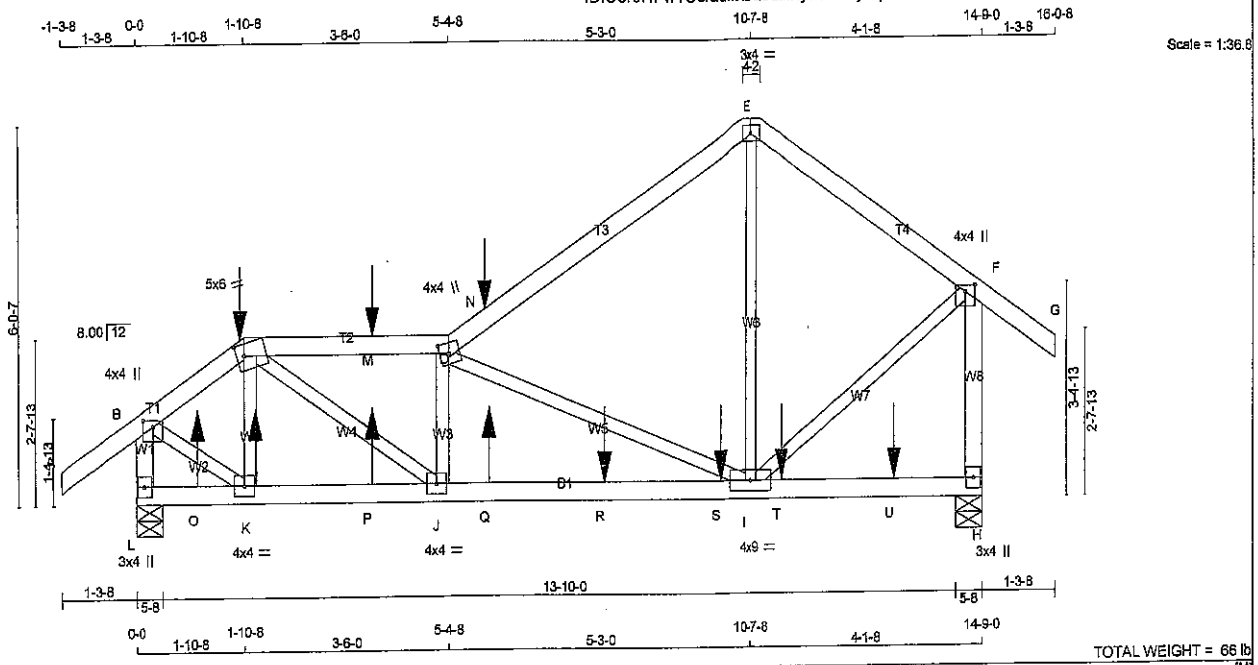

TOTAL WEIGHT = 66 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTWW-m	MT20	5.0	6.0	2.25	1.50
D	TTWW+m	MT20	4.0	4.0	2.00	1.75
E	TTW-p	MT20	3.0	4.0		
F	TMVW+p	MT20	4.0	4.0	1.25	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMV1+p	MT20	3.0	4.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 2.4 lbs FACTORED DOWN AND 102.9 lbs FACTORED UP AT 1-10-8, AND 1.9 lbs FACTORED DOWN AND 103.4 lbs FACTORED UP AT 4-0-12, AND 1.9 lbs FACTORED DOWN AND 91.4 lbs FACTORED UP AT 6-0-12 ON TOP CHORD, AND 6.5 lbs FACTORED UP AT 1-0-12, 6.5 lbs FACTORED UP AT 2-0-12, 6.5 lbs FACTORED UP AT 4-0-12, 6.5 lbs FACTORED UP AT 6-0-12, 3.7 lbs FACTORED DOWN AT 8-0-12, 112.8 lbs FACTORED DOWN AT 10-0-12, AND 112.8 lbs FACTORED DOWN AT 11-2-4, AND 3.7 lbs FACTORED DOWN AT 13-2-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	996	0	996	0	0	5-8	5-8
H	1101	0	1101	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	795	460 / 0	158 / 0	0 / 0	0 / 0	158 / 0	0 / 0
H	864	550 / 0	158 / 0	0 / 0	0 / 0	158 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.36 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (7)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.3	-84.3	0.12 (1)	10.00
B- C	-758 / 0	-84.3	-84.3	0.13 (1)	6.25
C- M	-1307 / 0	-84.3	-84.3	0.22 (1)	5.36
M- D	-1307 / 0	-84.3	-84.3	0.22 (1)	5.36
D- N	-650 / 0	-84.3	-84.3	0.47 (1)	6.25
N- E	-650 / 0	-84.3	-84.3	0.47 (1)	6.25
E- F	-851 / 0	-84.3	-84.3	0.28 (1)	6.25
F- G	0 / 32	-84.3	-84.3	0.12 (1)	10.00
L- B	-980 / 0	0.0	0.0	0.11 (1)	7.81
H- F	-1055 / 0	0.0	0.0	0.22 (1)	7.63
L- O	0 / 0	-28.0	-28.0	0.04 (7)	10.00
O- K	0 / 0	-28.0	-28.0	0.04 (7)	10.00
K- P	0 / 597	-28.0	-28.0	0.20 (7)	10.00
P- J	0 / 597	-28.0	-28.0	0.20 (7)	10.00
J- Q	0 / 1324	-28.0	-28.0	0.42 (1)	10.00
Q- R	0 / 1324	-28.0	-28.0	0.42 (1)	10.00
R- S	0 / 1324	-28.0	-28.0	0.42 (1)	10.00
S- I	0 / 1324	-28.0	-28.0	0.42 (1)	10.00
I- T	0 / 0	-28.0	-28.0	0.21 (2)	10.00
T- U	0 / 0	-28.0	-28.0	0.21 (2)	10.00
U- H	0 / 0	-28.0	-28.0	0.21 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-265 / 0	0.05 (1)
C- J	0 / 863	0.21 (1)
J- D	-380 / 0	0.07 (1)
D- I	-856 / 0	0.51 (1)
I- E	0 / 334	0.08 (2)
B- K	0 / 693	0.17 (1)
I- F	0 / 676	0.17 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	1-10-8	-1	-2	103	BACK	VERT	TOTAL
K	2-0-12	6	—	6	BACK	VERT	TOTAL
M	4-0-12	-1	-2	103	BACK	VERT	TOTAL
N	6-0-12	-1	-2	91	BACK	VERT	TOTAL
O	1-0-12	6	—	6	BACK	VERT	TOTAL
P	4-0-12	6	—	6	BACK	VERT	TOTAL
Q	6-0-12	6	—	6	BACK	VERT	TOTAL
R	8-0-12	-4	-4	—	BACK	VERT	TOTAL
S	10-0-12	-113	-113	—	BACK	VERT	TOTAL
T	11-2-4	-113	-113	—	BACK	VERT	TOTAL
U	13-2-4	-4	-4	—	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.49")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.49")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.47 (D-E:1) , BC=0.42 (I-J:1) , WB=0.51 (D-I:1) , SSI=0.17 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (E) (INPUT = 0.90)
JSI METAL= 0.29 (O) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM45798-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285475	T123	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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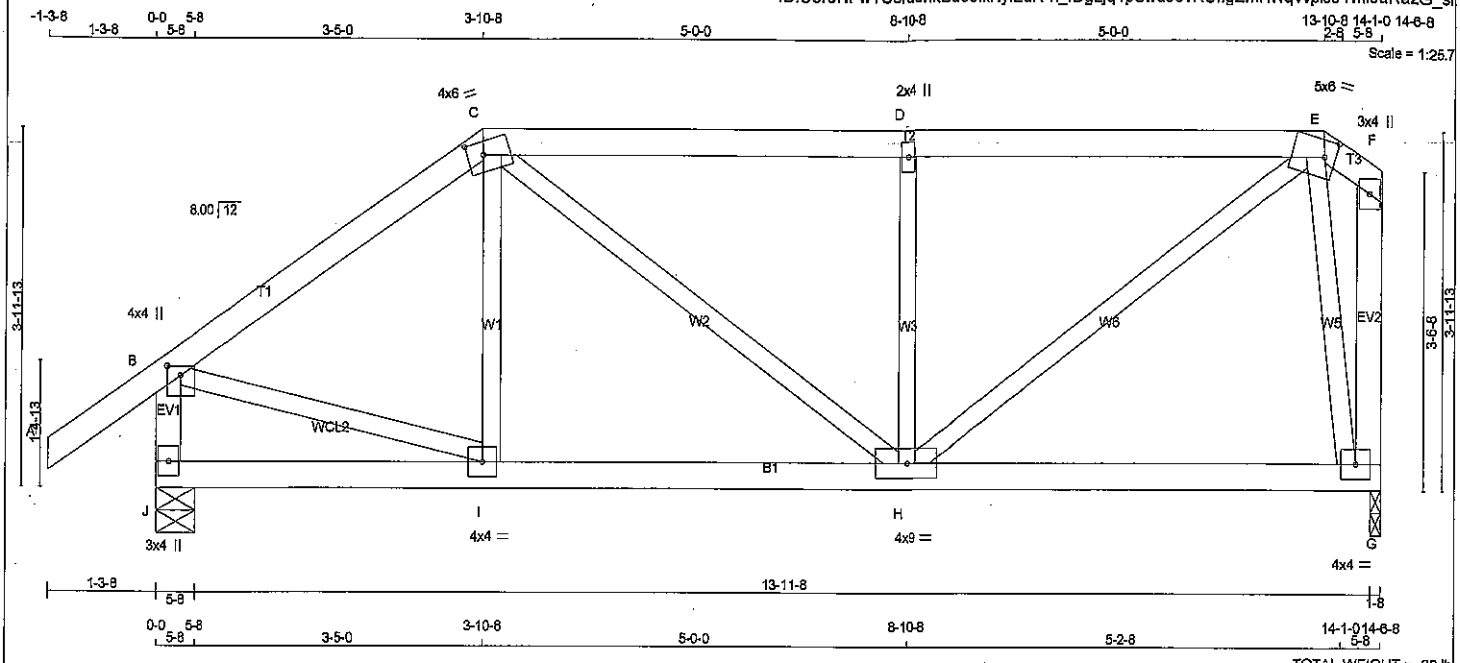

TOTAL WEIGHT = 62 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTWW-m	MT20	4.0	6.0	1.75	2.25
D	TMW+w	MT20	2.0	4.0		
E	TTWW-m	MT20	5.0	6.0	2.25	1.50
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	932	0	932	0	0	5-8	5-8
G	816	0	816	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	749	442 / 0	153 / 0	0 / 0	0 / 0	154 / 0	0 / 0
G	670	372 / 0	153 / 0	0 / 0	0 / 0	145 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.09 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	I- C	-47 / 131	0.03 (3)	
B- C	-778 / 0	-84.3	-84.3	0.23 (1)	6.25	C- H	0 / 258	0.06 (1)	
C- D	-652 / 0	-84.3	-84.3	0.36 (1)	6.09	H- D	-516 / 0	0.13 (1)	
D- E	-852 / 0	-84.3	-84.3	0.38 (1)	6.09	B- I	0 / 669	0.15 (1)	
E- F	0 / 0	-84.3	-84.3	0.01 (1)	10.00	E- G	-734 / 0	0.18 (1)	
J- B	-887 / 0	0.0	0.0	0.09 (1)	7.81	H- E	0 / 896	0.20 (1)	
G- F	-28 / 0	0.0	0.0	0.01 (1)	7.81				
J- I	0 / 0	-28.0	-28.0	0.11 (3)	10.00				
I- H	0 / 645	-28.0	-28.0	0.26 (2)	10.00				
H- G	0 / 131	-28.0	-28.0	0.21 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.48")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.48")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.36/1.00 (D-E:1) , BC=0.28/1.00 (H-I:2) , WB=0.20/1.00 (E-H:1) , SSI=0.20/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1667	822	2284	1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (E) (INPUT = 0.90)
JSI METAL= 0.24 (I) (INPUT = 1.00)

DWG NO. TAM26866-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286040	T124A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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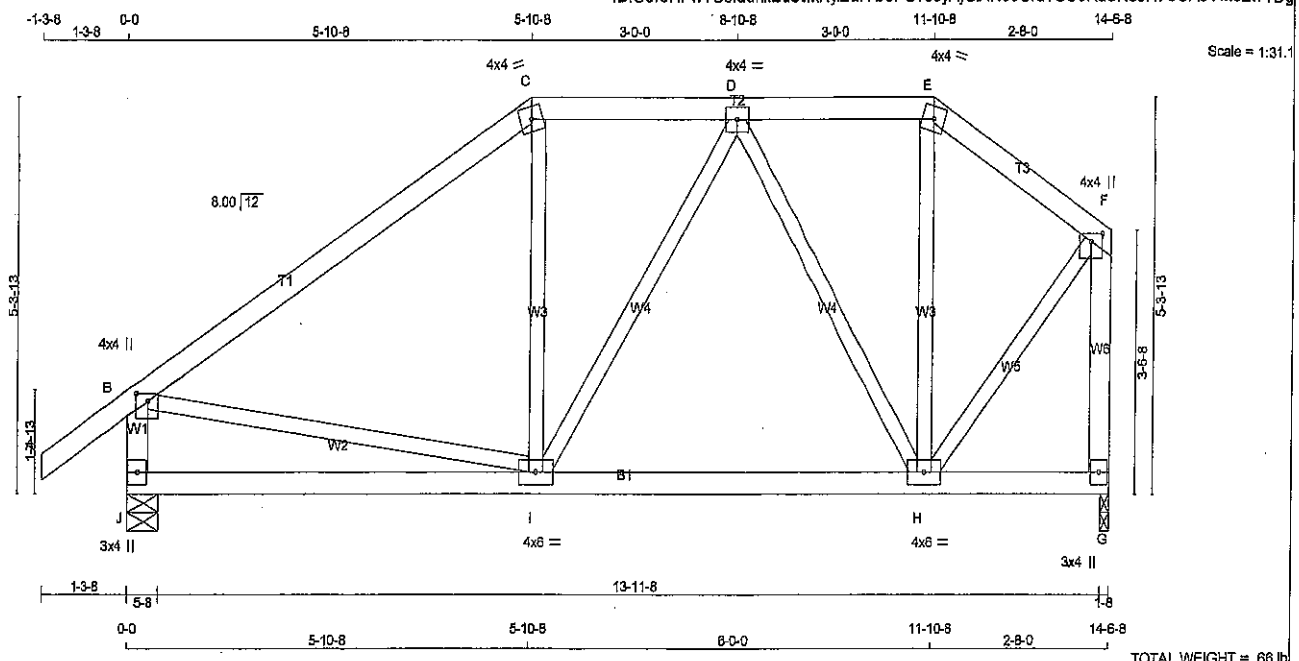

TOTAL WEIGHT = 66 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTW-m	MT20	4.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TTW-m	MT20	4.0	4.0		
F	TMVW+p	MT20	4.0	4.0	1.25	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	6.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	932	0	932	0	0	5-8	5-8
G	816	0	816	0	0	1-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	749	442 / 0	153 / 0	0 / 0	0 / 0	154 / 0	0 / 0
G	570	372 / 0	153 / 0	0 / 0	0 / 0	145 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)		MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM	TO			FR-TO			
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	I- C	0 / 161	0.04 (3)	
B- C	-711 / 0	-84.3	-84.3	0.38 (1)	6.25	I- D	0 / 114	0.03 (3)	
C- D	-593 / 0	-84.3	-84.3	0.10 (1)	6.25	D- H	-398 / 0	0.23 (1)	
D- E	-372 / 0	-84.3	-84.3	0.09 (1)	6.25	H- E	0 / 113	0.03 (3)	
E- F	-448 / 0	-84.3	-84.3	0.08 (1)	6.25	B- I	0 / 600	0.14 (1)	
J- B	-684 / 0	0.0	0.0	0.09 (1)	7.81	H- F	0 / 570	0.13 (1)	
G- F	-794 / 0	0.0	0.0	0.17 (1)	7.81				
J- I	0 / 0	-28.0	-28.0	0.26 (3)	10.00				
I- H	0 / 571	-28.0	-28.0	0.32 (2)	10.00				
H- G	0 / 0	-28.0	-28.0	0.12 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(65 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.46")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.46")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.38/1.00 (B-C:1) , BC=0.32/1.00 (H-I:2) , WB=0.23/1.00 (D-H:1) , SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (I) (INPUT = 0.90)
JSI METAL= 0.21 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
1136
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM332B-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286040	T125A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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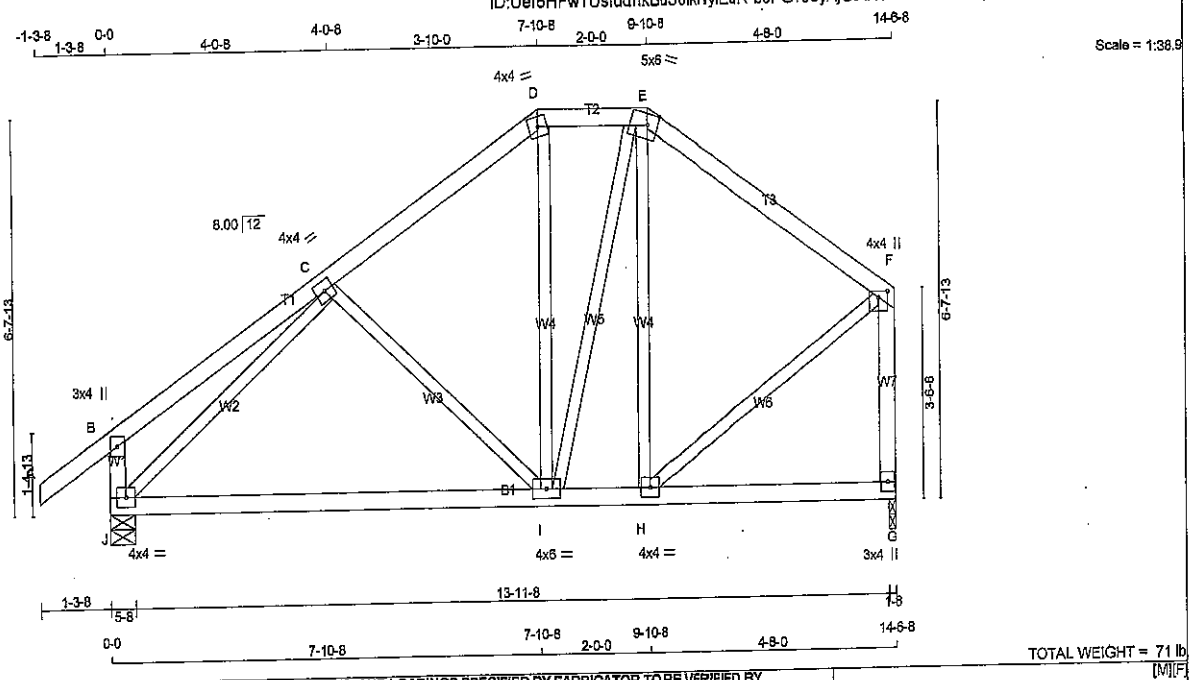

TOTAL WEIGHT = 71 lb [MiTek]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW-m	MT20	4.0	4.0		
E	TTWW-m	MT20	5.0	6.0	2.25	2.25
F	TMVW+p	MT20	4.0	4.0	1.25	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	816	0	816	0	0	1-8	1-8
J	932	0	932	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	670	372 / 0	153 / 0	0 / 0	0 / 0	145 / 0	0 / 0
J	749	442 / 0	153 / 0	0 / 0	0 / 0	154 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00
B- C	0 / 21	-84.3	-84.3	0.21 (1)	10.00
C- D	-607 / 0	-84.3	-84.3	0.16 (1)	6.25
D- E	-489 / 0	-84.3	-84.3	0.05 (1)	6.25
E- F	-503 / 0	-84.3	-84.3	0.24 (1)	6.25
J- B	-242 / 0	0.0	0.0	0.02 (1)	7.81
G- F	-758 / 0	0.0	0.0	0.16 (1)	7.81
J- I	0 / 636	-28.0	-28.0	0.48 (2)	10.00
I- H	0 / 413	-28.0	-28.0	0.47 (2)	10.00
H- G	0 / 0	-28.0	-28.0	0.15 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- I	-202 / 30	0.10 (1)
I- D	0 / 173	0.04 (2)
I- E	0 / 276	0.06 (2)
H- E	-252 / 0	0.18 (1)
J- C	-874 / 0	0.41 (1)
H- F	0 / 504	0.11 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.48")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.48")
CALCULATED VERT. DEFL.(TL) = L/ 722 (0.24")

CSI: TC=0.24/1.00 (E-F:1) , BC=0.48/1.00 (I-J:2) , WB=0.41/1.00 (C-J:1) , SSI=0.18/1.00 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (J) (INPUT = 0.90)
JSI METAL= 0.30 (C) (INPUT = 1.00)

DWG NO. TAM 3329-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286040	T126Z7	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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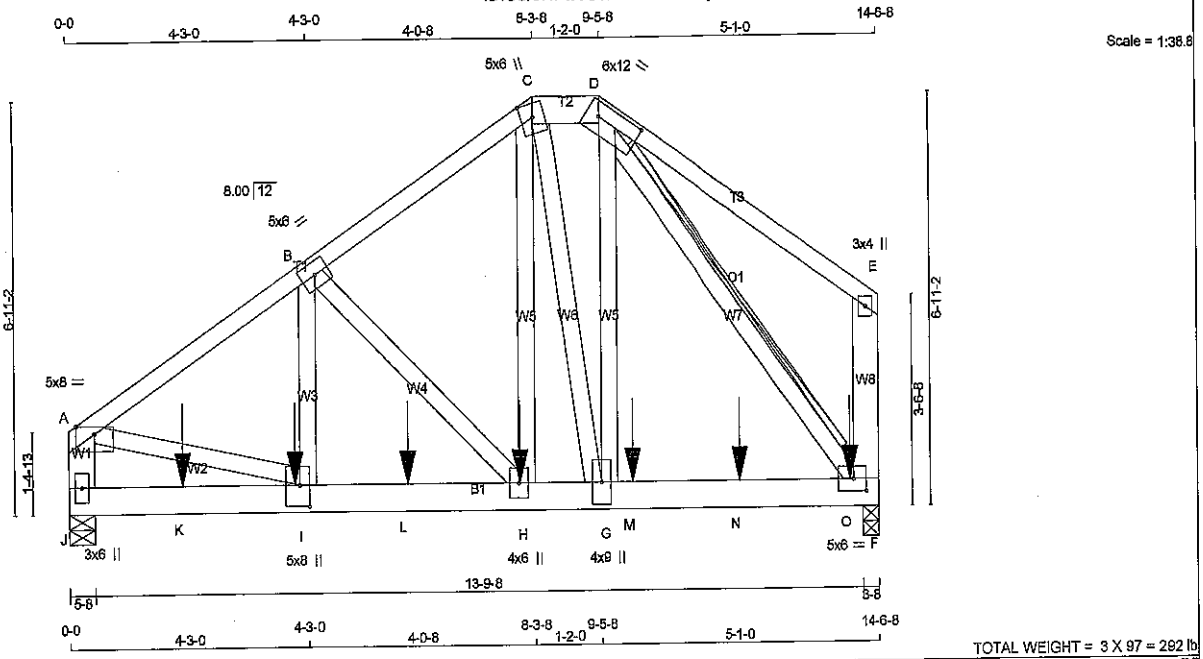

TOTAL WEIGHT = 3 X 97 = 292 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x6	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
J - A	2x6	DRY	No.2	SPF
F - E	2x6	DRY	No.2	SPF
J - F	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
D- E	1	12	TOP
C- D	2	12	TOP
J- A	2	12	TOP
F- E	2	6	SIDE(91.4)
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
J- F	2	7	SIDE(790.3)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	8.0	Edge	
B	TMWW-t	MT20	5.0	6.0		
C	TTWW+m	MT20	5.0	6.0	2.75	2.50
D	TTWW-h	MT20	6.0	12.0	2.75	9.25
E	TMV+p	MT20	3.0	4.0		
F	BMVW1-t	MT20	5.0	6.0	2.50	2.75
G	BMWW+t	MT20	4.0	9.0		
H	BMWW+t	MT20	4.0	6.0		
I	BMWW+t	MT20	5.0	8.0	4.25	2.00
J	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	7814	0	7814	0	0	5-8	5-8
F	9343	0	9343	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	6337	3647 / 0	1357 / 0	0 / 0	0 / 0	1333 / 0	0 / 0
F	7605	4330 / 0	1660 / 0	0 / 0	0 / 0	1615 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.93 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x6 DRY SPF No.2 T-BRACE AT D-F

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-8931 / 0	-84.3	-84.3	0.20 (1)		3.93
B- C	-8077 / 0	-84.3	-84.3	0.12 (1)		4.68
C- D	-4870 / 0	-84.3	-84.3	0.01 (1)		6.18
D- E	0 / 0	-84.3	-84.3	0.12 (1)		10.00
J- A	-6862 / 0	0.0	0.0	0.15 (1)		6.77
F- E	-214 / 0	0.0	0.0	0.01 (1)		7.81
J- K	0 / 0	-28.0	-28.0	0.23 (1)		10.00
K- I	0 / 0	-28.0	-28.0	0.23 (1)		10.00
I- L	0 / 7442	-28.0	-28.0	0.31 (1)		10.00
L- H	0 / 7442	-28.0	-28.0	0.31 (1)		10.00
H- G	0 / 4978	-28.0	-28.0	0.38 (1)		10.00
G- M	0 / 5008	-28.0	-28.0	0.38 (1)		10.00
M- N	0 / 5008	-28.0	-28.0	0.38 (1)		10.00
N- O	0 / 5008	-28.0	-28.0	0.38 (1)		10.00
O- F	0 / 5008	-28.0	-28.0	0.38 (1)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- B	0 / 3565	0.19 (1)
B- H	-3341 / 0	0.38 (1)
H- C	0 / 3678	0.20 (1)
C- G	-543 / 0	0.10 (1)
G- D	0 / 6139	0.33 (1)
A- I	0 / 7644	0.41 (1)
D- F	-8127 / 0	0.60 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	STYLE
H	8-0-4	-2266	-2266	—	FRONT	VERT	TOTAL
I	4-0-4	-2266	-2266	—	FRONT	VERT	TOTAL
K	2-0-4	-2266	-2266	—	FRONT	VERT	TOTAL
L	6-0-4	-2266	-2266	—	FRONT	VERT	TOTAL
M	10-0-4	-2152	-2152	—	FRONT	VERT	TOTAL
N	12-0-4	-2152	-2152	—	FRONT	VERT	TOTAL
O	14-0-4	-2158	-2158	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.46")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.46")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.20/1.00 (A-B:1) , BC=0.38/1.00 (F-G:1) , WB=0.60/1.00 (D-F:1) , SSI=0.78/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SSI GRIP= 0.90 (D) (INPUT = 0.90)
SSI METAL= 0.65 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER 1318 S. KATSOULAKOS PROVINCE OF ONTARIO

P6/K

DWG NO. TAM 3330-18

STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
286040	T126Z7	1	3	TRUSS DESC.	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 2265.9 lbs FACTORED DOWN AT 2-0-4, 2265.9 lbs FACTORED DOWN AT 4-0-4, 2265.9 lbs FACTORED DOWN AT 6-0-4, 2265.9 lbs FACTORED DOWN AT 8-0-4, 2151.6 lbs FACTORED DOWN AT 10-0-4, AND 2151.6 lbs FACTORED DOWN AT 12-0-4, AND 2158.3 lbs FACTORED DOWN AT 14-0-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. YAM 3330 - 18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285473	T127	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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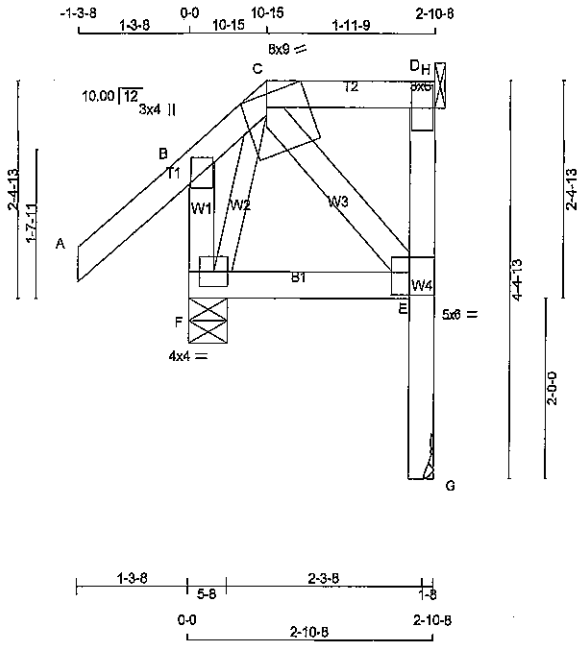


Scale = 1:25.6

TOTAL WEIGHT = 2 X 19 = 38 lb

(M)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - H	2x4	DRY	No.2	SPF
G - D	2x4	DRY	No.2	SPF
F - B	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
F - C	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TTWW-m	MT20	8.0	9.0	Edge	3.25
D	TMV1+t	MT20	3.0	6.0		
E	SVMW-I	MT20	5.0	6.0	3.00	2.50
F	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	116	0	116	0	0	1-8	1-8
G	25	0	38	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
F	301	0	301	0	0	5-8	5-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	90	59 / -9	15 / 0	0 / 0	0 / 0	16 / 0	0 / 0
G	29	2 / -6	16 / 0	0 / 0	0 / 0	11 / 0	0 / 0
F	227	159 / 0	30 / 0	0 / 0	0 / 0	38 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

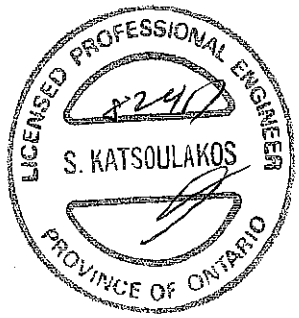
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 37	-84.3	-84.3	0.12 (1)	10.00	C-E	-13 / 35	0.01 (5)
B-C	-58 / 0	-84.3	-84.3	0.11 (1)	6.25	F-C	-19 / 60	0.01 (5)
C-D	0 / 2	-84.3	-84.3	0.05 (1)	10.00			
D-H	0 / 0	-76.8	-76.8	0.00 (1)	10.00			
G-E	-38 / 0	0.0	0.0	0.01 (2)	7.81			
E-D	-1 / 37	0.0	0.0	0.01 (2)	7.81			
F-B	-244 / 0	0.0	0.0	0.03 (1)	7.81			
F-E	-24 / 7	-28.0	-28.0	0.07 (3)	6.25			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.12 (A-B:1) , BC=0.07 (E-F:3) , WB=0.01 (C-F:5) , SSI=0.08 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (F) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)

DWG NO. TAM 4196517

STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285473	T128	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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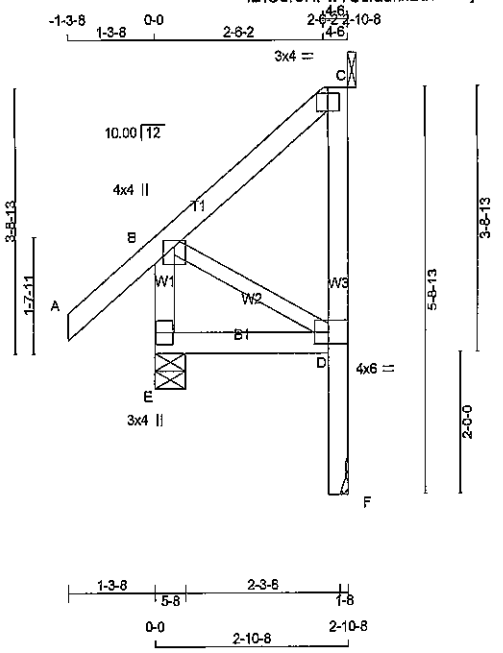


Scale = 1:32.8

TOTAL WEIGHT = 2 X 19 = 38 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
F - C	2x4	DRY	No.2	SPF
E - B	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TVM1-p	MT20	3.0	4.0	Edge	2.00
D	BVMW-I	MT20	4.0	6.0	2.00	2.50
E	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
C	136	0	136	0	0	1-8	1-8
F	25	0	45	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
E	278	0	278	0	0	5-8	5-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	101	74 / 0	11 / 0	0 / 0	0 / 0	16 / 0	0 / 0
F	32	0 / 0	19 / 0	0 / 0	0 / 0	13 / 0	0 / 0
E	212	144 / 0	30 / 0	0 / 0	0 / 0	37 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, E

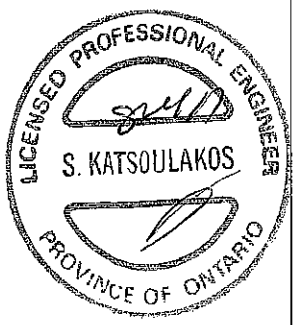
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX. (PLF) CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 37	-84.3	-84.3	0.12 (1)	10.00	B- D	0 / 0	0.00 (1)
B- C	0 / 0	-84.3	-84.3	0.12 (1)	10.00			
F- D	-45 / 0	0.0	0.0	0.01 (3)	7.81			
D- C	0 / 26	0.0	0.0	0.00 (3)	10.00			
E- B	-238 / 0	0.0	0.0	0.03 (1)	7.81			
E- D	0 / 0	-28.0	-28.0	0.07 (3)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.12 (B-C:1) , BC=0.07 (D-E:3) , WB=0.00 (B-D:1) , SSI=0.07 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (B) (INPUT = 0.90)
JSI METAL= 0.06 (C) (INPUT = 1.00)

DWG NO. TAM 41966-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285473	T129	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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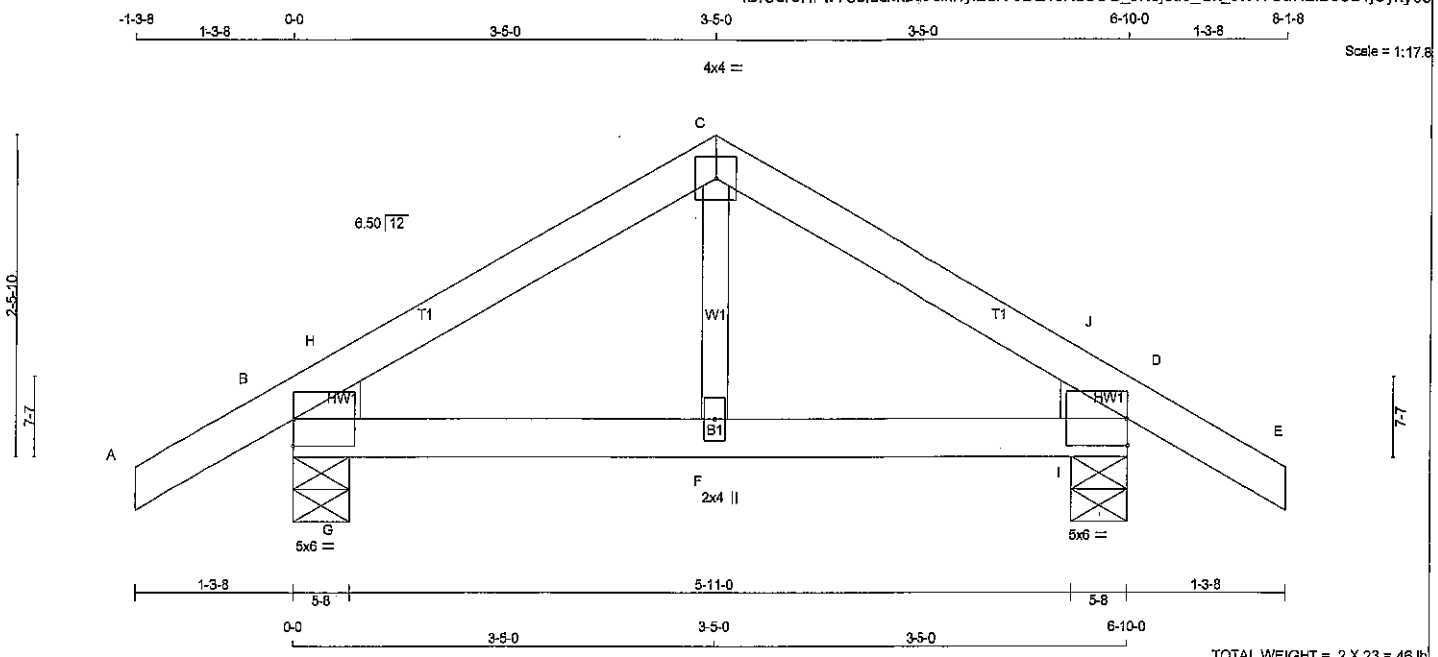

TOTAL WEIGHT = 2 X 23 = 46 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-I	MT20	5.0	6.0		
C	TTW-p	MT20	4.0	4.0		
D	TMBH1-I	MT20	5.0	6.0		
F	BMW+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
B	498	0	498	0	0	5-8	5-8	2x4 L
D	498	0	498	0	0	5-8	5-8	2x4 R

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	393	244 / 0	72 / 0	0 / 0	0 / 0	77 / 0	0 / 0
D	393	244 / 0	72 / 0	0 / 0	0 / 0	77 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 17	-84.3	-84.3	0.11 (1)	10.00	F- C	0 / 176	0.04 (2)
B- H	-431 / 0	-84.3	-84.3	0.06 (1)	6.25	G- H	-42 / 121	0.00 (1)
H- C	-377 / 0	-84.3	-84.3	0.09 (1)	6.25	I- J	-42 / 121	0.00 (1)
C- J	-377 / 0	-84.3	-84.3	0.09 (1)	6.25			
J- D	-431 / 0	-84.3	-84.3	0.06 (1)	6.25			
D- E	0 / 17	-84.3	-84.3	0.11 (1)	10.00			
B- G	0 / 324	-28.0	-28.0	0.09 (1)	10.00			
G- F	0 / 324	-28.0	-28.0	0.11 (1)	10.00			
F- I	0 / 324	-28.0	-28.0	0.11 (1)	10.00			
I- D	0 / 324	-28.0	-28.0	0.09 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.23")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.11 (D-E:1) , BC=0.11 (F-G:1) , WB=0.04 (C-F:2) , SSI=0.10 (C-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (D) (INPUT = 0.90)
JSI METAL= 0.11 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM41967-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285473	T129Z	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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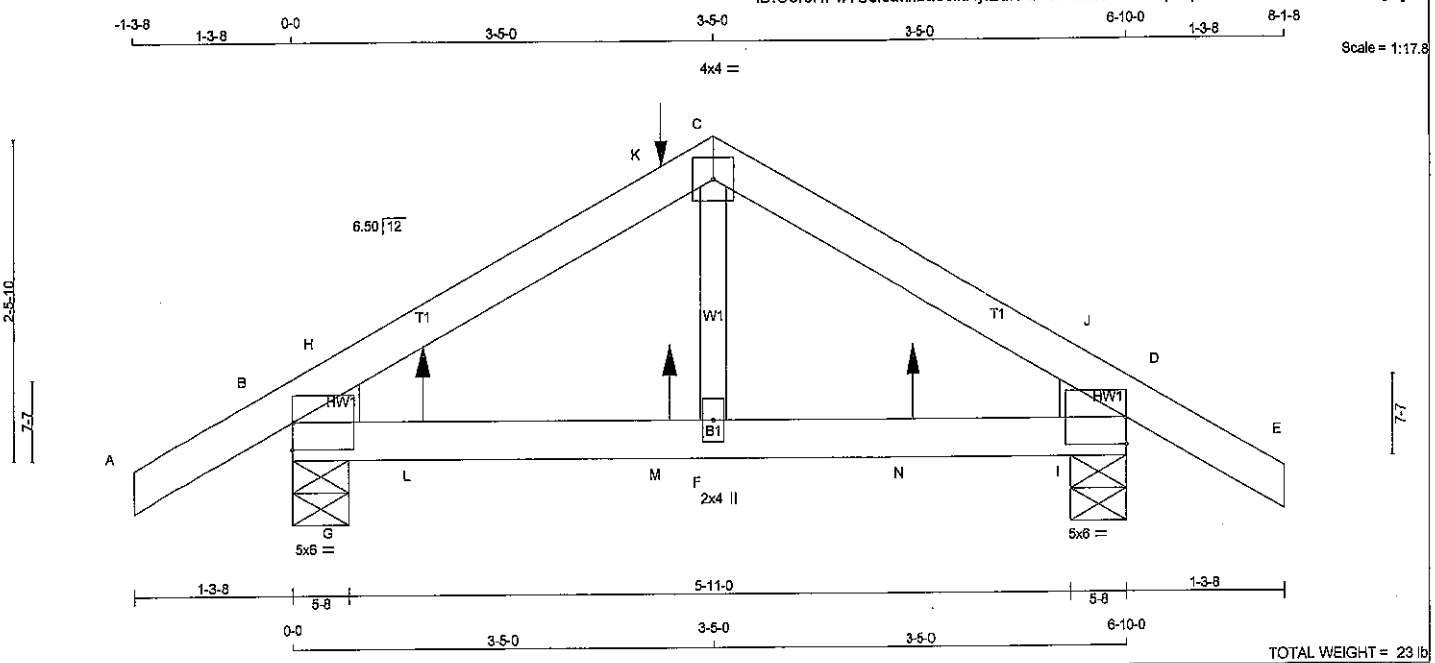


TOTAL WEIGHT = 23 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMBH1-I	MT20	5.0	6.0	
C	TTW-p	MT20	4.0	4.0	
D	TMBH1-I	MT20	5.0	6.0	
F	BMW+w	MT20	2.0	4.0	

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1.2 lbs FACTORED DOWN AND 84.7 lbs FACTORED UP AT 3-0-12 ON TOP CHORD, AND 6.2 lbs FACTORED UP AT 1-0-12, AND 6.2 lbs FACTORED UP AT 3-0-12, AND 6.2 lbs FACTORED UP AT 5-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
B	488	0	488	0	0	5-8	5-8	2x4 L
D	490	0	490	0	0	5-8	5-8	2x4 R

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	386	238 / 0	72 / 0	0 / 0	0 / 0	77 / 0	0 / 0
D	386	239 / 0	72 / 0	0 / 0	0 / 0	77 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (7)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 17	-84.3	-84.3	0.12 (1)	10.00		F- C	0 / 178	0.05 (7)
B- H	-415 / 0	-84.3	-84.3	0.08 (1)	6.25		G- H	-51 / 121	0.00 (1)
H- K	-366 / 0	-84.3	-84.3	0.11 (1)	6.25		I- J	-68 / 121	0.00 (1)
K- C	-366 / 0	-84.3	-84.3	0.11 (1)	6.25				
C- J	-366 / 0	-84.3	-84.3	0.11 (5)	6.25				
J- D	-415 / 0	-84.3	-84.3	0.08 (1)	6.25				
D- E	0 / 17	-84.3	-84.3	0.12 (1)	10.00				
B- G	0 / 315	-28.0	-28.0	0.10 (1)	10.00				
G- L	0 / 315	-28.0	-28.0	0.12 (5)	10.00				
L- M	0 / 315	-28.0	-28.0	0.12 (5)	10.00				
M- F	0 / 315	-28.0	-28.0	0.12 (5)	10.00				
F- N	0 / 315	-28.0	-28.0	0.12 (5)	10.00				
N- I	0 / 315	-28.0	-28.0	0.12 (5)	10.00				
I- D	0 / 315	-28.0	-28.0	0.10 (5)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE
K	3-0-12	-0	-1	85	FRONT	VERT	TOTAL
L	1-0-12	6	—	6	FRONT	VERT	TOTAL
M	3-0-12	6	—	6	FRONT	VERT	TOTAL
N	5-0-12	6	—	6	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP. CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH..	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.23")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.12 (D-E:1) , BC=0.12 (F-I:5) , WB=0.05 (C-F:7) , SSI=0.11 (C-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

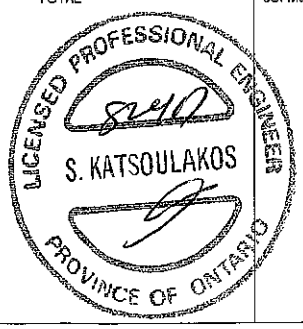
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (D) (INPUT = 0.90)
JSI METAL= 0.10 (D) (INPUT = 1.00)



DWG NO. TAM 41968-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285474	T130A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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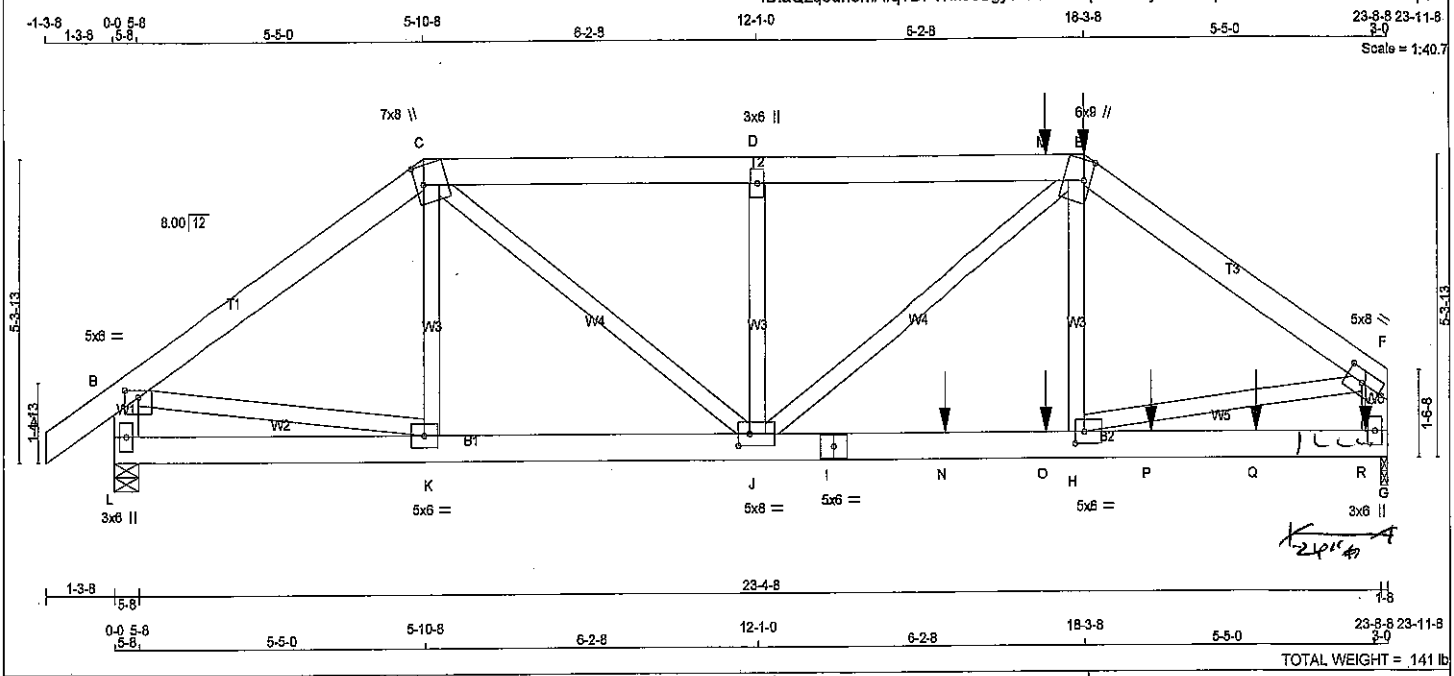

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - F	2x6	DRY	No.2	SPF
L - B	2x6	DRY	No.2	SPF
G - F	2x6	DRY	No.2	SPF
L - I	2x6	DRY	2100F 1.8E	SPF
I - G	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TTWW+m	MT20	7.0	8.0	4.00	1.75
D	TMW+w	MT20	3.0	6.0		
E	TTWW+m	MT20	6.0	9.0	4.25	1.50
F	TMVW-t	MT20	5.0	8.0	2.50	3.75
G	BMV1+p	MT20	3.0	6.0		
H	BMWW-t	MT20	5.0	6.0	2.50	2.00
I	BS-t	MT20	5.0	6.0		
J	BMWWW-t	MT20	5.0	6.0	2.50	2.50
K	BMWW-t	MT20	5.0	6.0		
L	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 355.0 lbs FACTORED DOWN AT 18-3-8, AND 112.8 lbs FACTORED DOWN AT 17-6-12 ON TOP CHORD, AND 1105.6 lbs FACTORED DOWN AT 15-7-8, 71.7 lbs FACTORED DOWN AT 17-6-12, 71.7 lbs FACTORED DOWN AT 19-6-12, AND 71.7 lbs FACTORED DOWN AT 21-6-12, AND 86.6 lbs FACTORED DOWN AT 23-6-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

SCAB JOINT # G S=# WITH 2 X 6 #2 SPF 24" LONG 1 SIDE(S) USING 2 ROW(S) 3 1/4" ~~ARC GUN~~ COIL/COMMON WIRE NAILS AT 3" O/C STAGGERED (15 NAILS/SCAB) REQUIRED FOR 1 1/4" SRG PROVISION.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1986	0	1986	0	0	5-8	5-8
G	2569	0	2569	0	0	1-6	1-6

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1604	934 / 0	338 / 0	0 / 0	0 / 0	334 / 0	0 / 0
G	2116	1163 / 0	490 / 0	0 / 0	0 / 0	463 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.38 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF) CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 33	-84.3	-84.3	0.07 (1)	10.00
B- C	-2252 / 0	-84.3	-84.3	0.32 (1)	5.08
C- D	-3126 / 0	-84.3	-84.3	0.34 (1)	4.43
D- M	-3126 / 0	-84.3	-84.3	0.34 (1)	4.43
M- E	-3126 / 0	-84.3	-84.3	0.34 (1)	4.43
E- F	-3232 / 0	-84.3	-84.3	0.34 (1)	4.38
L- B	-1900 / 0	0.0	0.0	0.13 (1)	7.25
G- F	-2517 / 0	0.0	0.0	0.18 (1)	8.48
L- K	0 / 0	-28.0	-28.0	0.07 (2)	10.00
K- J	0 / 1865	-28.0	-28.0	0.29 (1)	10.00
J- I	0 / 2701	-28.0	-28.0	0.62 (1)	10.00
I- N	0 / 2701	-28.0	-28.0	0.62 (1)	10.00
N- O	0 / 2701	-28.0	-28.0	0.62 (1)	10.00
O- H	0 / 2701	-28.0	-28.0	0.62 (1)	10.00
H- P	0 / 0	-28.0	-28.0	0.26 (1)	10.00
P- Q	0 / 0	-28.0	-28.0	0.26 (1)	10.00
Q- R	0 / 0	-28.0	-28.0	0.26 (1)	10.00
R- G	0 / 0	-28.0	-28.0	0.26 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-192 / 147	0.05 (1)
C- J	0 / 1599	0.28 (1)
J- D	-628 / 0	0.17 (1)
J- E	0 / 539	0.10 (1)
H- E	0 / 702	0.12 (2)
B- K	0 / 1691	0.33 (1)
H- F	0 / 2726	0.48 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
E	18-3-8	-33	-36	—	FRONT	VERT	DEAD
E	18-3-8	-323	-323	—	FRONT	VERT	SNOW
M	17-6-12	-113	-113	—	BACK	VERT	TOTAL
N	15-7-8	-1106	-1106	—	BACK	VERT	TOTAL
O	17-6-12	-41	-72	—	BACK	VERT	TOTAL
P	19-6-12	-41	-72	—	BACK	VERT	TOTAL
Q	21-6-12	-41	-72	—	BACK	VERT	TOTAL
R	23-6-12	-49	-87	—	BACK	VERT	TOTAL

DWG NO. TAM B306-18
STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER 2018 S. KATSOULAKOS PROVINCE OF ONTARIO

[M]

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.80")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.80")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.23")

CSI: TC=0.34/1.00 (D-E:1) , BC=0.62/1.00 (H-J:1) , WB=0.48/1.00 (F-H:1) , SSI=0.53/1.00 (H-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	622	2264	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.67 (H) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285474	T131A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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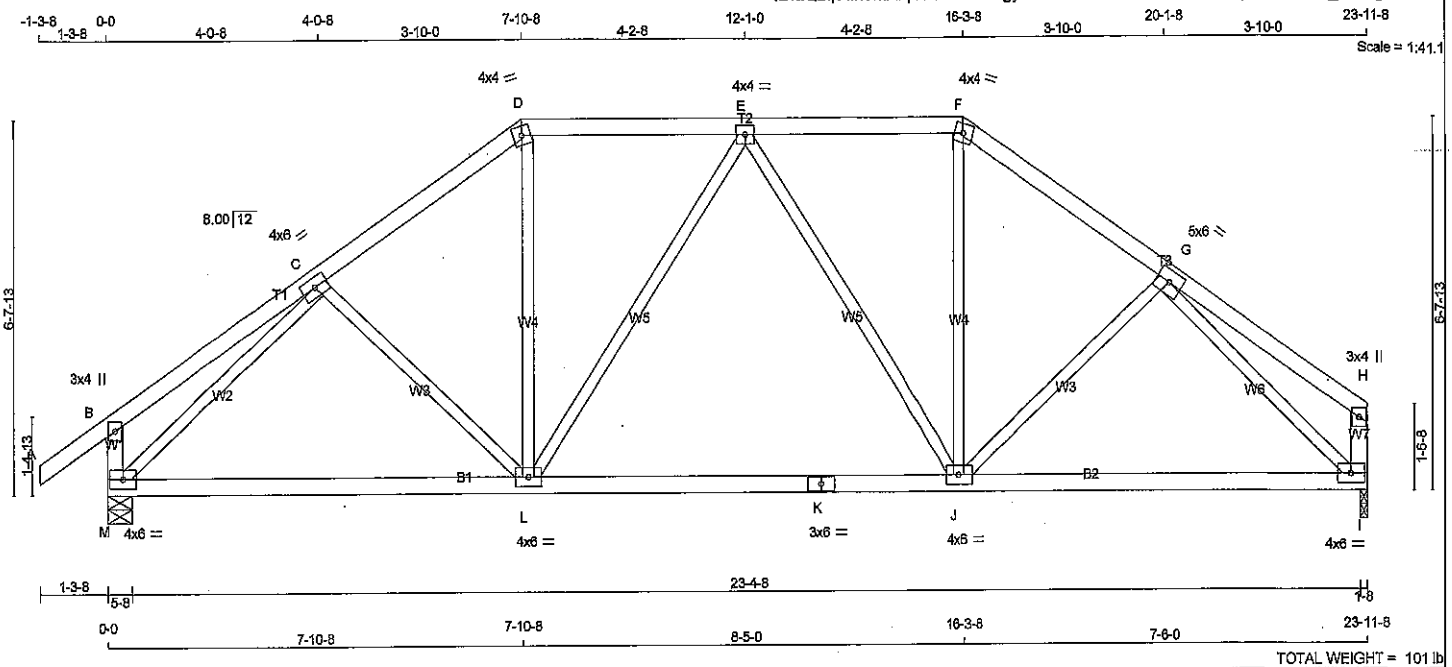


TOTAL WEIGHT = 101 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMV+p	MT20	3.0	4.0	
C	TMWW-t	MT20	4.0	6.0	
D	TTW-m	MT20	4.0	4.0	
E	TMWW-t	MT20	4.0	4.0	
F	TTW-m	MT20	4.0	4.0	
G	TMWW-t	MT20	5.0	6.0	
H	TMV+p	MT20	3.0	4.0	
I	BMVW1-t	MT20	4.0	6.0	
J	BMWWW-t	MT20	4.0	6.0	
K	BS-t	MT20	3.0	6.0	
L	BMWWW-t	MT20	4.0	6.0	
M	BMVW1-t	MT20	4.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1460	0	1460	0	0	5-8	5-8
I	1345	0	1345	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1183	683 / 0	252 / 0	0 / 0	0 / 0	248 / 0	0 / 0
I	1104	613 / 0	252 / 0	0 / 0	0 / 0	240 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.34 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	C- L	-68 / 98	0.03 (1)
B- C	0 / 20	-84.3	-84.3	0.20 (1)	10.00	L- D	0 / 498	0.11 (1)
C- D	-1399 / 0	-84.3	-84.3	0.18 (1)	5.34	L- E	-240 / 0	0.28 (1)
D- E	-1153 / 0	-84.3	-84.3	0.20 (1)	5.71	E- J	-270 / 0	0.31 (1)
E- F	-1137 / 0	-84.3	-84.3	0.20 (1)	5.74	J- F	0 / 485	0.11 (1)
F- G	-1379 / 0	-84.3	-84.3	0.17 (1)	5.36	J- G	-20 / 116	0.03 (3)
G- H	0 / 20	-84.3	-84.3	0.19 (1)	10.00	M- C	-1642 / 0	0.78 (1)
M- B	-245 / 0	0.0	0.0	0.03 (1)	7.81	G- I	-1613 / 0	0.71 (1)
I- H	-121 / 0	0.0	0.0	0.01 (1)	7.81			
M- L	0 / 1198	-28.0	-28.0	0.58 (2)	10.00			
L- K	0 / 1264	-28.0	-28.0	0.58 (2)	10.00			
K- J	0 / 1264	-28.0	-28.0	0.58 (2)	10.00			
J- I	0 / 1144	-28.0	-28.0	0.54 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.60")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.60")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.25")

CSI: TC=0.20/1.00 (D-E:1) , BC=0.58/1.00 (J-L:2) , WB=0.78/1.00 (C-M:1) , SSI=0.18/1.00 (L-M:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

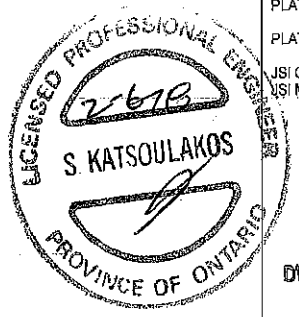
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (M) (INPUT = 0.90)
JSI METAL= 0.40 (C) (INPUT = 1.00)



DWG NO. TAM 8062-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285474	T132A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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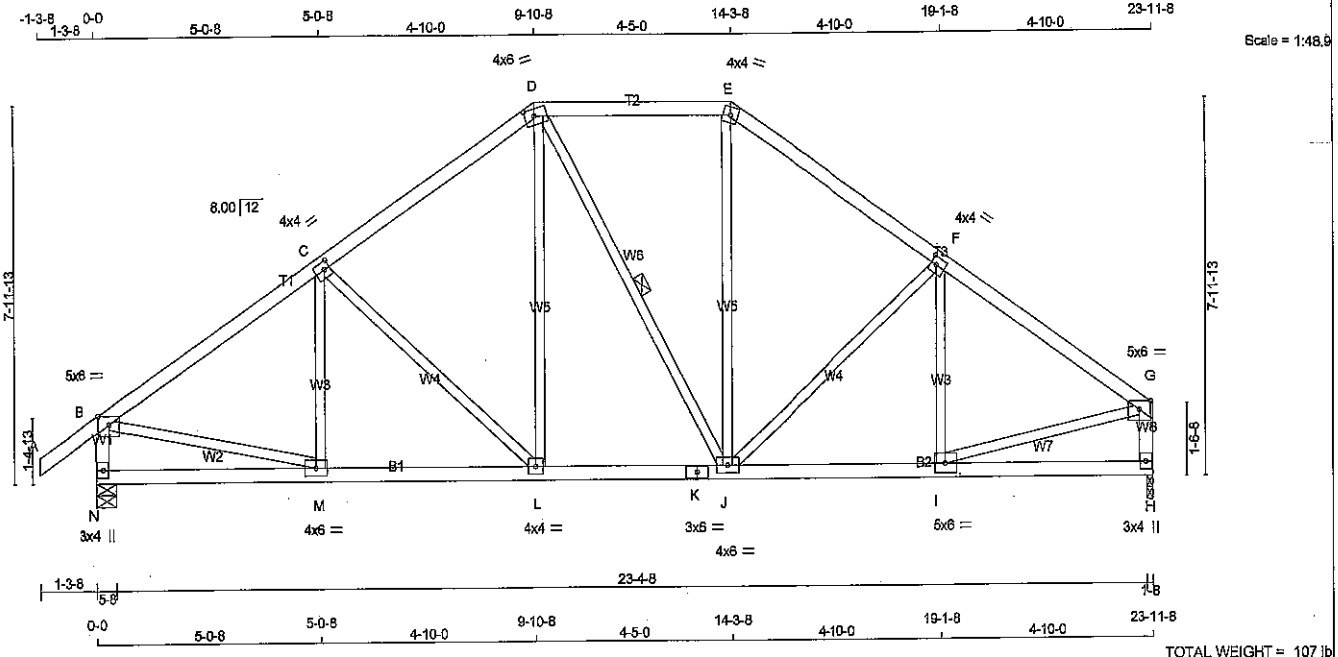

TOTAL WEIGHT = 107 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	4.0	6.0	1.75	2.50
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0	2.00	1.50
G	TMVW-p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	5.0	6.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BS-t	MT20	3.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMWW-t	MT20	4.0	6.0		
N	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REORD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
N	1460	0	1460	0	0	5-8	5-8
H	1345	0	1345	0	0	1-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1183	683 / 0	252 / 0	0 / 0	0 / 0	248 / 0	0 / 0
H	1104	613 / 0	252 / 0	0 / 0	0 / 0	240 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.05 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX. (PLF) CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00
B- C	-1504 / 0	-84.3	-84.3	0.29 (1)	5.05
C- D	-1253 / 0	-84.3	-84.3	0.26 (1)	5.43
D- E	-1017 / 0	-84.3	-84.3	0.22 (1)	5.95
E- F	-1245 / 0	-84.3	-84.3	0.27 (1)	5.46
F- G	-1456 / 0	-84.3	-84.3	0.28 (1)	5.13
N- B	-1402 / 0	0.0	0.0	0.14 (1)	6.88
H- G	-1269 / 0	0.0	0.0	0.13 (1)	7.10
N- M	0 / 0	-28.0	-28.0	0.17 (3)	10.00
M- L	0 / 1274	-28.0	-28.0	0.32 (2)	10.00
L- K	0 / 1023	-28.0	-28.0	0.23 (2)	10.00
K- J	0 / 1023	-28.0	-28.0	0.23 (2)	10.00
J- I	0 / 1234	-28.0	-28.0	0.30 (2)	10.00
I- H	0 / 0	-28.0	-28.0	0.16 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- C	-117 / 143	0.04 (1)
C- L	-344 / 0	0.28 (1)
L- D	0 / 374	0.08 (2)
D- J	-11 / 0	0.01 (1)
J- E	0 / 364	0.08 (2)
J- F	-297 / 0	0.24 (1)
I- F	-161 / 115	0.05 (1)
B- M	0 / 1303	0.29 (1)
I- G	0 / 1272	0.29 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.60")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.80")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.29/1.00 (B-C:1) , BC=0.32/1.00 (L-M:2) , WB=0.29/1.00 (B-M:1) , SSI=0.17/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (M) (INPUT = 0.90)
JSI METAL= 0.32 (M) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 8063 -R8
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285474	T133A	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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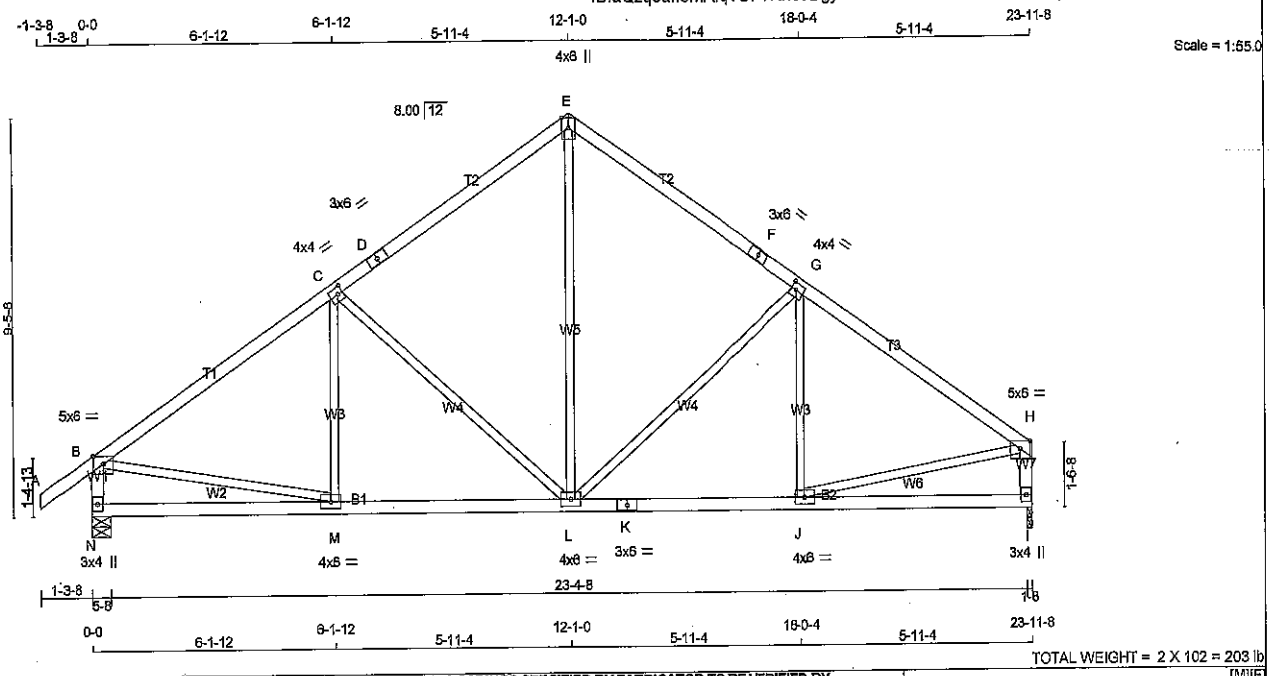

TOTAL WEIGHT = 2 X 102 = 203 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTW+p	MT20	4.0	6.0	Edge	
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW-p	MT20	5.0	6.0	Edge	
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	4.0	6.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	4.0	6.0		
M	BMWW-t	MT20	4.0	6.0		
N	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1460	0	1460	0	0	5-8	5-8
I	1345	0	1345	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1183	683 / 0	252 / 0	0 / 0	0 / 0	248 / 0	0 / 0
I	1104	613 / 0	252 / 0	0 / 0	0 / 0	240 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.85 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	L- E	0 / 809	0.18 (1)
B- C	-1501 / 0	-84.3	-84.3	0.46 (1)	4.85	L- G	-474 / 0	0.61 (1)
C- D	-1104 / 0	-84.3	-84.3	0.42 (1)	5.49	J- G	-71 / 202	0.05 (3)
D- E	-1104 / 0	-84.3	-84.3	0.42 (1)	5.49	C- L	-514 / 0	0.66 (1)
E- F	-1103 / 0	-84.3	-84.3	0.41 (1)	5.52	M- C	-36 / 225	0.05 (3)
F- G	-1103 / 0	-84.3	-84.3	0.41 (1)	5.52	B- M	0 / 1297	0.29 (1)
G- H	-1465 / 0	-84.3	-84.3	0.43 (1)	4.93	J- H	0 / 1273	0.29 (1)
N- B	-1390 / 0	0.0	0.0	0.14 (1)	6.90			
I- H	-1277 / 0	0.0	0.0	0.13 (1)	7.13			
N- M	0 / 0	-28.0	-28.0	0.25 (3)	10.00			
M- L	0 / 1277	-28.0	-28.0	0.40 (2)	10.00			
L- K	0 / 1247	-28.0	-28.0	0.38 (2)	10.00			
K- J	0 / 1247	-28.0	-28.0	0.38 (2)	10.00			
J- I	0 / 0	-28.0	-28.0	0.24 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	48.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.80")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.80")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.46/1.00 (B-C:1) , BC=0.40/1.00 (L-M:2) , WB=0.66/1.00 (C-L:1) , SSI=0.21/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (J) (INPUT = 0.90)
JSI METAL= 0.34 (K) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM B064-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285474	T133AC	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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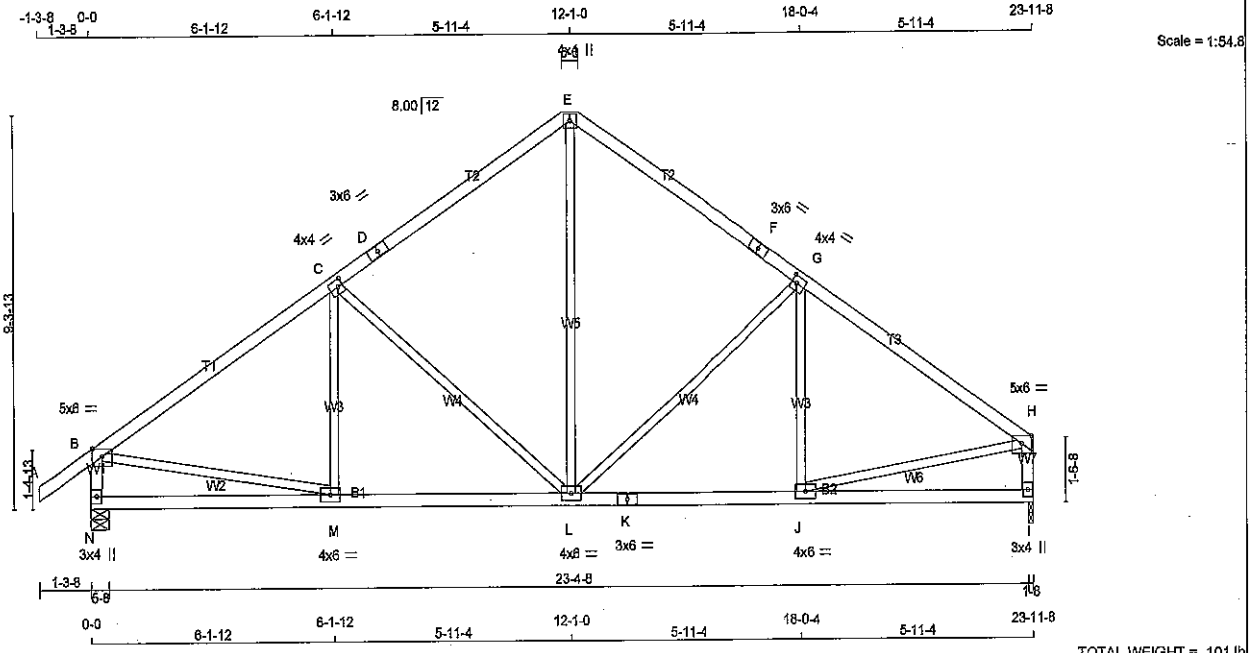

TOTAL WEIGHT = 101 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTW+p	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW-p	MT20	5.0	6.0	Edge	
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	4.0	6.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	4.0	6.0		
M	BMWW-t	MT20	4.0	6.0		
N	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1460	0	1460	0	0	5-8	5-8
I	1345	0	1345	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1183	683 / 0	252 / 0	0 / 0	0 / 0	248 / 0	0 / 0
I	1104	613 / 0	252 / 0	0 / 0	0 / 0	240 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.85 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	CSI	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.3	-84.3	0.11 (1)		10.00
B- C	-1501 / 0	-84.3	-84.3	0.46 (1)		4.85
C- D	-1104 / 0	-84.3	-84.3	0.42 (1)		5.49
D- E	-1104 / 0	-84.3	-84.3	0.42 (1)		5.49
E- F	-1103 / 0	-84.3	-84.3	0.41 (1)		5.52
F- G	-1103 / 0	-84.3	-84.3	0.41 (1)		5.52
G- H	-1465 / 0	-84.3	-84.3	0.43 (1)		4.93
N- B	-1390 / 0	0.0	0.0	0.14 (1)		6.90
I- H	-1277 / 0	0.0	0.0	0.13 (1)		7.13
N- M	0 / 0	-28.0	-28.0	0.25 (3)		10.00
M- L	0 / 1277	-26.0	-28.0	0.40 (2)		10.00
L- K	0 / 1247	-28.0	-28.0	0.38 (2)		10.00
K- J	0 / 1247	-28.0	-28.0	0.38 (2)		10.00
J- I	0 / 0	-28.0	-28.0	0.24 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI
L- E	0 / 809	0.18 (1)
L- G	-474 / 0	0.61 (1)
J- G	-71 / 202	0.05 (3)
C- L	-514 / 0	0.66 (1)
M- C	-36 / 225	0.05 (3)
B- M	0 / 1297	0.29 (1)
J- H	0 / 1273	0.29 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.60")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.60")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.46/1.00 (B-C:1) , BC=0.40/1.00 (L-M:2) , WB=0.66/1.00 (C-L:1) , SSI=0.21/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (J) (INPUT = 0.90)
JSI METAL= 0.34 (K) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 8065 -08
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285474	T134	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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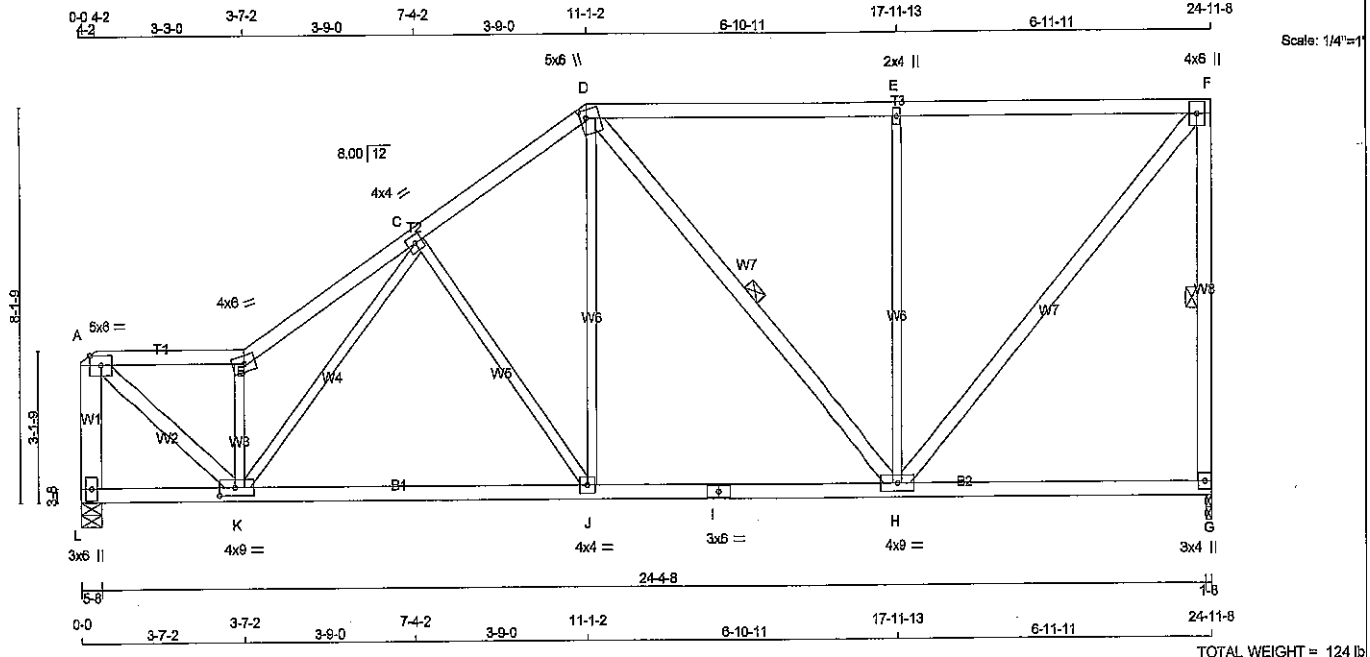

TOTAL WEIGHT = 124 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - A	2x6	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
A - K	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0	Edge	
B	TTW-m	MT20	4.0	6.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWWW-t	MT20	4.0	9.0	2.00	4.00
L	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1401	0	1401	0	0	1-8	1-8
L	1401	0	1401	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1150	639 / 0	262 / 0	0 / 0	0 / 0	250 / 0	0 / 0
L	1150	639 / 0	262 / 0	0 / 0	0 / 0	250 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.68 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	-1513 / 0	-84.3	-84.3	0.22 (1)		5.10
B- C	-1853 / 0	-84.3	-84.3	0.25 (1)		4.68
C- D	-1362 / 0	-84.3	-84.3	0.23 (1)		5.30
D- E	-977 / 0	-84.3	-84.3	0.76 (1)		4.93
E- F	-977 / 0	-84.3	-84.3	0.76 (1)		4.93
G- F	-1321 / 0	0.0	0.0	0.39 (1)		5.83
L- A	-1376 / 0	0.0	0.0	0.14 (1)		7.81
L- K	0 / 0	-28.0	-28.0	0.25 (3)		10.00
K- J	0 / 1376	-28.0	-28.0	0.46 (2)		10.00
J- I	0 / 1121	-28.0	-28.0	0.45 (2)		10.00
I- H	0 / 1121	-28.0	-28.0	0.45 (2)		10.00
H- G	0 / 0	-28.0	-28.0	0.32 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI
K- B	-1313 / 0	0.24 (1)
C- J	-446 / 0	0.35 (1)
J- D	0 / 569	0.13 (1)
A- K	0 / 1951	0.31 (1)
H- F	0 / 1471	0.24 (1)
D- H	-217 / 0	0.12 (1)
H- E	-725 / 0	0.93 (1)
K- C	0 / 345	0.08 (2)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.83")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.83")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.76/1.00 (E-F:1) , BC=0.46/1.00 (J-K:2) , WB=0.93/1.00 (E-H:1) , SSI=0.28/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1667	622	2284	1856

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (K) (INPUT = 0.90)
JSI METAL= 0.45 (B) (INPUT = 1.00)

[M]

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM B066 -18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285474	T135	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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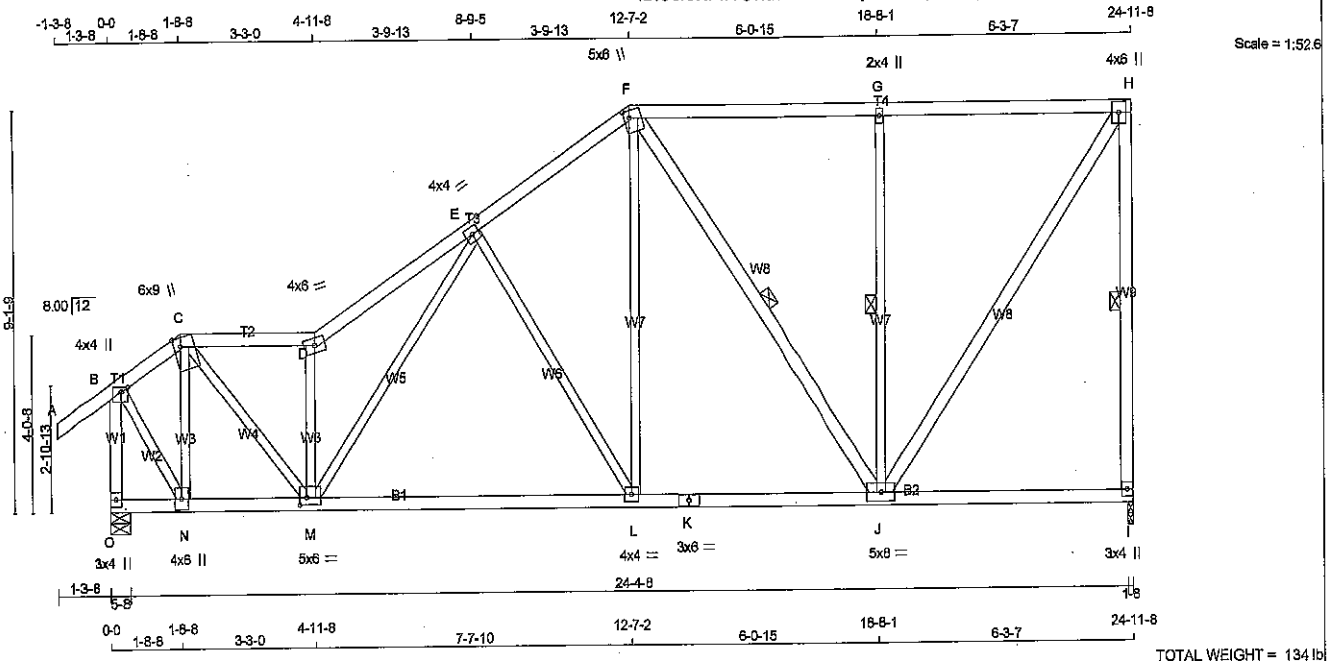


LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
F - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	1.50
C	TTWW+m	MT20	6.0	9.0	Edge	2.00
D	TTW-m	MT20	4.0	6.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTWW+m	MT20	5.0	8.0	2.00	1.50
G	TMW+w	MT20	2.0	4.0		
H	TMVW+p	MT20	4.0	6.0		
I	BMV1+p	MT20	3.0	4.0		
J	BMWWW-t	MT20	5.0	8.0		
K	BS-t	MT20	3.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMWWW-t	MT20	5.0	8.0	2.00	2.00
N	BMWW+t	MT20	4.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	1398	0	1398	0	0	1-8	1-8
O	1520	0	1520	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1148	637 / 0	262 / 0	0 / 0	0 / 0	249 / 0	0 / 0
O	1231	711 / 0	262 / 0	0 / 0	0 / 0	258 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.74 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

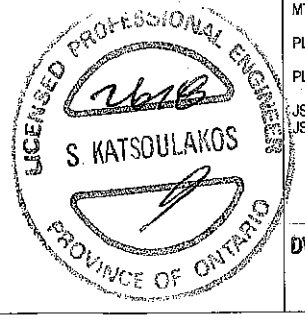
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF H-I, F-J, G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.3	-84.3	0.11 (1)		10.00
B- C	-751 / 0	-84.3	-84.3	0.11 (1)		6.25
C- D	-1471 / 0	-84.3	-84.3	0.18 (1)		5.21
D- E	-1785 / 0	-84.3	-84.3	0.26 (1)		4.74
E- F	-1218 / 0	-84.3	-84.3	0.23 (1)		5.53
F- G	-793 / 0	-84.3	-84.3	0.58 (1)		5.81
G- H	-793 / 0	-84.3	-84.3	0.58 (1)		5.81
I- H	-1326 / 0	0.0	0.0	0.51 (1)		5.62
O- B	-1494 / 0	0.0	0.0	0.23 (1)		6.71
O- N	0 / 0	-28.0	-28.0	0.04 (3)		10.00
N- M	0 / 578	-28.0	-28.0	0.33 (2)		10.00
M- L	0 / 1274	-28.0	-28.0	0.44 (2)		10.00
L- K	0 / 1001	-28.0	-28.0	0.41 (2)		10.00
K- J	0 / 1001	-28.0	-28.0	0.41 (2)		10.00
J- I	0 / 0	-28.0	-28.0	0.24 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- C	-817 / 0	0.20 (1)
C- M	0 / 1386	0.31 (1)
M- D	-1255 / 0	0.31 (1)
M- E	0 / 440	0.10 (1)
E- L	-523 / 0	0.56 (1)
L- F	0 / 845	0.15 (1)
F- J	-364 / 0	0.22 (1)
J- G	-646 / 0	0.35 (1)
J- H	0 / 1378	0.22 (1)
B- N	0 / 1045	0.24 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	48.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.83")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.83")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.58/1.00 (G-H:1) , BC=0.44/1.00 (L-M:2) , WB=0.56/1.00 (E-L:1) , SSI=0.25/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (M) (INPUT = 0.90)
JSI METAL= 0.44 (D) (INPUT = 1.00)

DWG NO. TAM 8067-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285474	T136	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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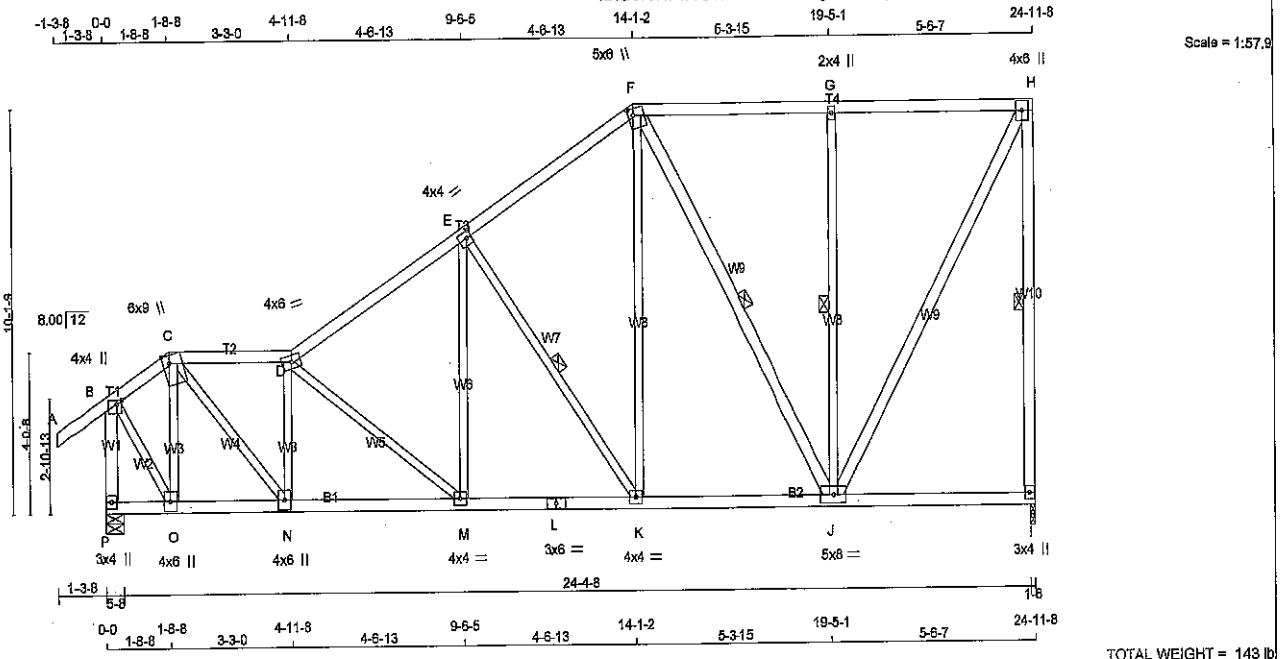


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
P - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
F - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	1.50
C	TTWW+m	MT20	6.0	9.0	Edge	2.00
D	TTWW-m	MT20	4.0	6.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.50
F	TTWW+m	MT20	5.0	6.0	2.00	1.50
G	TMW+w	MT20	2.0	4.0		
H	TMVW+p	MT20	4.0	6.0		
I	BMV1+p	MT20	3.0	4.0		
J	BMWWW-t	MT20	5.0	8.0		
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMWW+t	MT20	4.0	6.0		
O	BMWW+t	MT20	4.0	6.0		
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
I	1398	0	1398	0	0	1-8	1-8
P	1520	0	1520	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1146	637 / 0	262 / 0	0 / 0	0 / 0	249 / 0	0 / 0
P	1231	711 / 0	262 / 0	0 / 0	0 / 0	256 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, P

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF H-I, E-K, F-J, G-J.

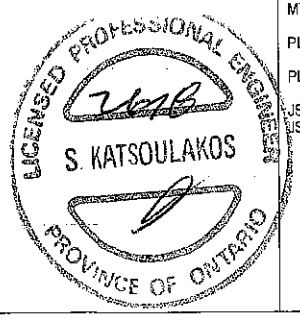
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 32	-84.3	-84.3	0.11 (1)		10.00
B- C	-758 / 0	-84.3	-84.3	0.11 (1)		5.25
C- D	-1451 / 0	-84.3	-84.3	0.18 (1)		5.24
D- E	-1475 / 0	-84.3	-84.3	0.34 (1)		5.00
E- F	-1072 / 0	-84.3	-84.3	0.32 (1)		5.88
F- G	-645 / 0	-84.3	-84.3	0.43 (1)		6.25
G- H	-645 / 0	-84.3	-84.3	0.43 (1)		8.25
I- H	-1336 / 0	0.0	0.0	0.67 (1)		5.60
P- B	-1506 / 0	0.0	0.0	0.23 (1)		8.66
P- O	0 / 0	-28.0	-28.0	0.04 (2)		10.00
O- N	0 / 585	-28.0	-28.0	0.14 (2)		10.00
N- M	0 / 1477	-28.0	-28.0	0.30 (1)		10.00
M- L	0 / 1250	-28.0	-28.0	0.27 (1)		10.00
L- K	0 / 1250	-28.0	-28.0	0.27 (1)		10.00
K- J	0 / 877	-28.0	-28.0	0.31 (2)		10.00
J- I	0 / 0	-28.0	-28.0	0.21 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- C	-781 / 0	0.19 (1)
C- N	0 / 1344	0.30 (1)
N- D	-928 / 0	0.23 (1)
D- M	-293 / 0	0.18 (1)
M- E	0 / 364	0.09 (2)
E- K	-670 / 0	0.31 (1)
K- F	0 / 665	0.15 (1)
F- J	-481 / 0	0.33 (1)
J- G	-556 / 0	0.40 (1)
J- H	0 / 1328	0.21 (1)
B- O	0 / 1055	0.24 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.83")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.83")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.67/1.00 (H-I:1) , BC=0.31/1.00 (J-K:2) , WB=0.40/1.00 (G-J:1) , SSI=0.22/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

USI GRIP= 0.90 (B) (INPUT = 0.90)
USI METAL= 0.37 (L) (INPUT = 1.00)

DWG NO. TAM 8068-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285474	T137	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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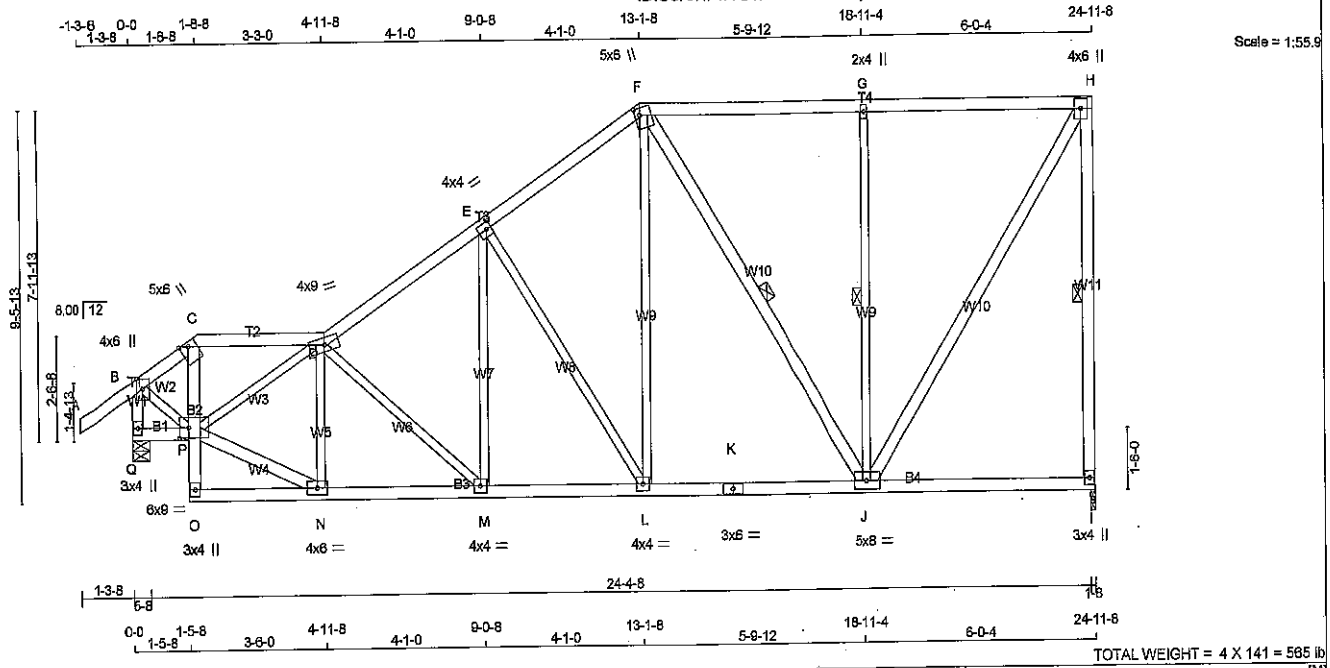


TOTAL WEIGHT = 4 X 141 = 565 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
Q - P	2x4	DRY	No.2	SPF
O - C	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
F - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMTV*+h	MT20	5.0	6.0	Edge	
D	TTWWW-m	MT20	4.0	9.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.50
F	TTWW+m	MT20	5.0	6.0	2.00	1.50
G	TMW+w	MT20	2.0	4.0		
H	TMVW+p	MT20	4.0	6.0		
I	BMV1+p	MT20	3.0	4.0		
J	BMWWW-t	MT20	5.0	8.0		
K	BS-t	MT20	3.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMWW-t	MT20	4.0	6.0		
O	BMV+p	MT20	3.0	4.0		
P	BVMWWW-I	MT20	6.0	9.0	3.00	2.75
Q	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	1398	0	1398	0	0	1-8	1-8
Q	1520	0	1520	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEA0	SOIL
I	1148	637 / 0	262 / 0	0 / 0	0 / 0	249 / 0	0 / 0
Q	1231	711 / 0	262 / 0	0 / 0	0 / 0	258 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, Q

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.05 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

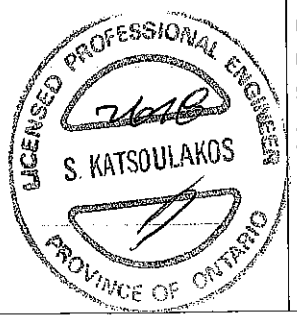
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF H-I, F-J, G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00
B-C	-1145 / 0	-84.3	-84.3	0.12 (1)	5.80
C-D	-891 / 0	-84.3	-84.3	0.17 (1)	6.25
D-E	-1508 / 0	-84.3	-84.3	0.27 (1)	5.05
E-F	-1160 / 0	-84.3	-84.3	0.25 (1)	5.60
F-G	-739 / 0	-84.3	-84.3	0.52 (1)	6.09
G-H	-739 / 0	-84.3	-84.3	0.52 (1)	6.09
I-H	-1331 / 0	0.0	0.0	0.57 (1)	5.62
O-B	-1497 / 0	0.0	0.0	0.15 (1)	6.71
Q-P	0 / 0	-28.0	-28.0	0.02 (2)	10.00
O-P	0 / 60	0.0	0.0	0.04 (1)	10.00
P-C	0 / 502	0.0	0.0	0.11 (1)	10.00
O-N	0 / 16	-28.0	-28.0	0.09 (3)	10.00
N-M	0 / 1480	-28.0	-28.0	0.28 (1)	10.00
M-L	0 / 1276	-28.0	-28.0	0.28 (1)	10.00
L-K	0 / 952	-28.0	-28.0	0.36 (2)	10.00
K-J	0 / 952	-28.0	-28.0	0.36 (2)	10.00
J-I	0 / 0	-28.0	-28.0	0.25 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P-N	0 / 1596	0.26 (1)
P-D	-690 / 0	0.18 (1)
N-D	-554 / 0	0.14 (1)
D-M	-277 / 0	0.14 (1)
M-E	0 / 346	0.08 (2)
E-L	-607 / 0	0.73 (1)
L-F	0 / 647	0.15 (1)
F-J	-396 / 0	0.25 (1)
J-G	-618 / 0	0.37 (1)
J-H	0 / 1359	0.22 (1)
B-P	0 / 1087	0.24 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.83")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.83")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.57/1.00 (H-I:1) , BC=0.36/1.00 (J-L:2) , WB=0.73/1.00 (E-L:1) , SSI=0.24/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (N) (INPUT = 0.90)
JSI METAL= 0.37 (N) (INPUT = 1.00)

DWG NO. TAM 8069-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
285474	T138X	1	1	43954 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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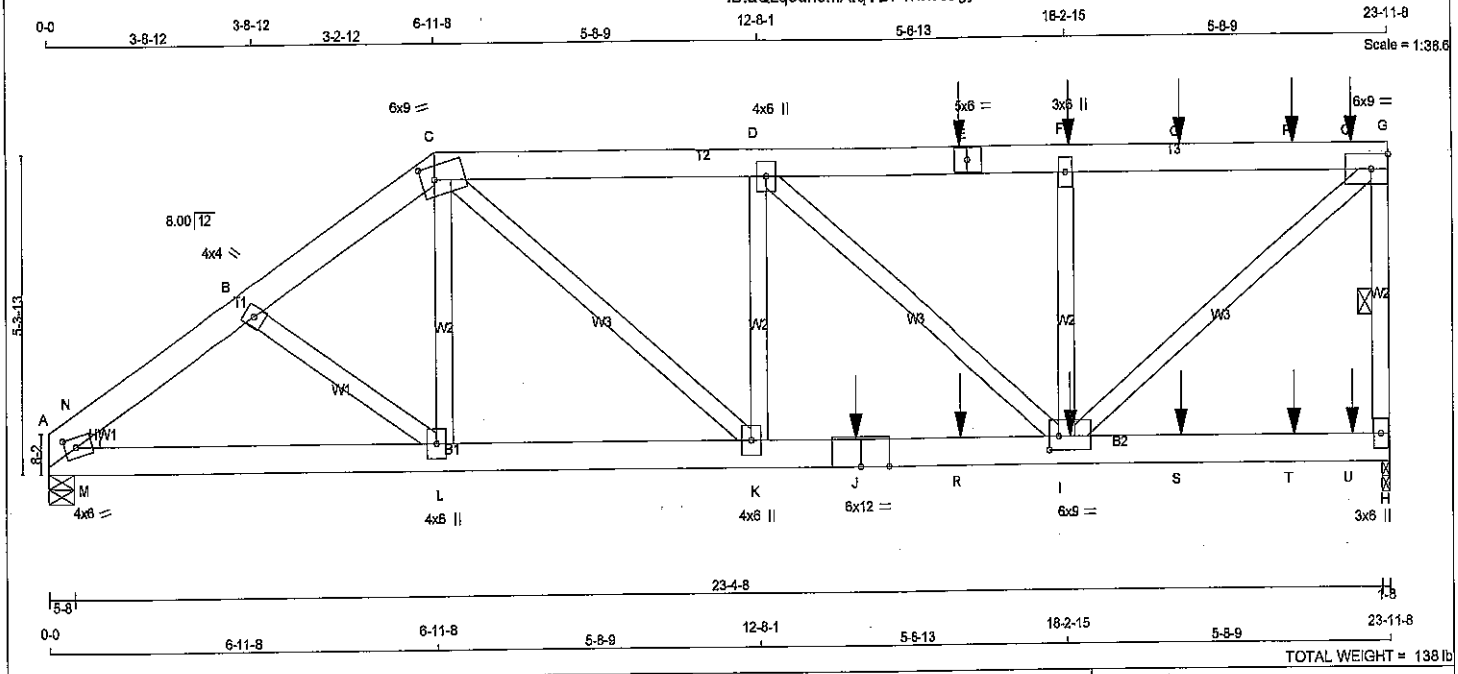

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - G	2x6	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
A - J	2x6	DRY	2100F 1.8E	SPF
J - H	2x6	DRY	2100F 1.8E	SPF
ALL WEBS	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBH1-m	MT20	4.0	6.0	2.00	2.50
B	TMW+w	MT20	4.0	4.0		
C	TTWW-m	MT20	6.0	9.0	2.75	2.75
D	TMWW+t	MT20	4.0	6.0		
E	TS-t	MT20	5.0	6.0		
F	TMW+w	MT20	3.0	6.0		
G	TMVW-t	MT20	6.0	9.0		Edge
H	BMV1+p	MT20	3.0	6.0		
I	BMWWW-t	MT20	6.0	9.0	2.75	2.00
J	BS-t	MT20	6.0	12.0		
K	BMWW+t	MT20	4.0	6.0		
L	BMWW+t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 101.4 lbs FACTORED DOWN AT 16-3-12, 101.4 lbs FACTORED DOWN AT 18-3-12, 101.4 lbs FACTORED DOWN AT 20-3-12, AND 101.4 lbs FACTORED DOWN AT 22-3-12, AND 116.4 lbs FACTORED DOWN AT 23-3-12 ON TOP CHORD, AND 1007.0 lbs FACTORED DOWN AT 14-4-8, 71.7 lbs FACTORED DOWN AT 16-3-12, 71.7 lbs FACTORED DOWN AT 18-3-12, 71.7 lbs FACTORED DOWN AT 20-3-12, AND 71.7 lbs FACTORED DOWN AT 22-3-12, AND 80.4 lbs FACTORED DOWN AT 23-3-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

SCAB JOINT # H & ___ WITH 2 X 6 #2 SPF 24" LONG 1 SIDE(S) USING 2 ROW(S) 3½" ARDOX SPIRAL/COMMON WIRE NAILS AT 3" O/C STAGGERED (12 NAILS/SCAB). REQUIRED FOR 1½" BRG PROVISION.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
H	2570	0	2570	0	0	1-8	1-8	
A	1659	0	1859	0	0	5-8	5-8	2x4 L

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	2116	1165 / 0	488 / 0	0 / 0	0 / 0	462 / 0	0 / 0
A	1524	850 / 0	345 / 0	0 / 0	0 / 0	330 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, A

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.53 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 MAX (PLF) FROM	TO	CSI (LC)	MAX. UNBRAC LENGTH
	A- N	-2993 / 0	-84.3	-84.3	0.11 (1)	4.75
	N- B	-2751 / 0	-84.3	-84.3	0.11 (1)	4.93
	B- C	-2530 / 0	-84.3	-84.3	0.12 (1)	5.08
	C- D	-3136 / 0	-84.3	-84.3	0.25 (1)	4.53
	D- E	-2447 / 0	-84.3	-84.3	0.32 (1)	4.90
	E- F	-2447 / 0	-84.3	-84.3	0.32 (1)	4.90
	F- O	-2447 / 0	-84.3	-84.3	0.36 (1)	4.66
	O- P	-2447 / 0	-84.3	-84.3	0.36 (1)	4.66
	P- Q	-2447 / 0	-84.3	-84.3	0.36 (1)	4.66
	Q- G	-2447 / 0	-84.3	-84.3	0.36 (1)	4.66
	H- G	-2468 / 0	0.0	0.0	0.37 (1)	4.32
	A- M	0 / 2274	-28.0	-28.0	0.16 (1)	10.00
	M- L	0 / 2274	-28.0	-28.0	0.17 (1)	10.00
	L- K	0 / 2104	-28.0	-28.0	0.30 (1)	10.00
	K- J	0 / 3136	-28.0	-28.0	0.53 (1)	10.00
	J- R	0 / 3136	-28.0	-28.0	0.53 (1)	10.00
	R- I	0 / 3136	-28.0	-28.0	0.53 (1)	10.00
	I- S	0 / 0	-28.0	-28.0	0.17 (2)	10.00
	S- T	0 / 0	-28.0	-28.0	0.17 (2)	10.00
	T- U	0 / 0	-28.0	-28.0	0.17 (2)	10.00
	U- H	0 / 0	-28.0	-28.0	0.17 (2)	10.00

WEBS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	MAX (LC)
B- L		-218 / 0	0.05 (1)
L- C		0 / 287	0.05 (2)
C- K		0 / 1352	0.24 (1)
K- D		0 / 239	0.04 (3)
D- I		-915 / 0	0.66 (1)
I- F		-844 / 0	0.23 (1)
I- G		0 / 3212	0.57 (1)
M- N		0 / 343	0.00 (1)

FACTORED CONCENTRATED LOADS (LBS)

	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
E	16-3-12	-101	-101	—	FRONT	VERT	TOTAL
F	18-3-12	-101	-101	—	FRONT	VERT	TOTAL
I	18-3-12	-41	-72	—	FRONT	VERT	TOTAL
J	14-4-8	-1007	-1007	—	FRONT	VERT	TOTAL
O	20-3-12	-101	-101	—	FRONT	VERT	TOTAL
P	22-3-12	-101	-101	—	FRONT	VERT	TOTAL
Q	23-3-12	-116	-116	—	FRONT	VERT	TOTAL
R	16-3-12	-41	-72	—	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.60")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.60")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.37/1.00 (G-H:1) , BC=0.53/1.00 (I-K:1) , WB=0.66/1.00 (D-I:1) , SSI=0.53/1.00 (I-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (A) (INPUT = 0.90)
JSI METAL= 0.95 (J) (INPUT = 1.00)

DWG NO. TAM 8070-18
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285474	T138X	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
S	20-3-12	-41	-72	---	FRONT	VERT	TOTAL
T	22-3-12	-41	-72	---	FRONT	VERT	TOTAL
U	23-3-12	-46	-80	---	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 8070-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285474	T139X	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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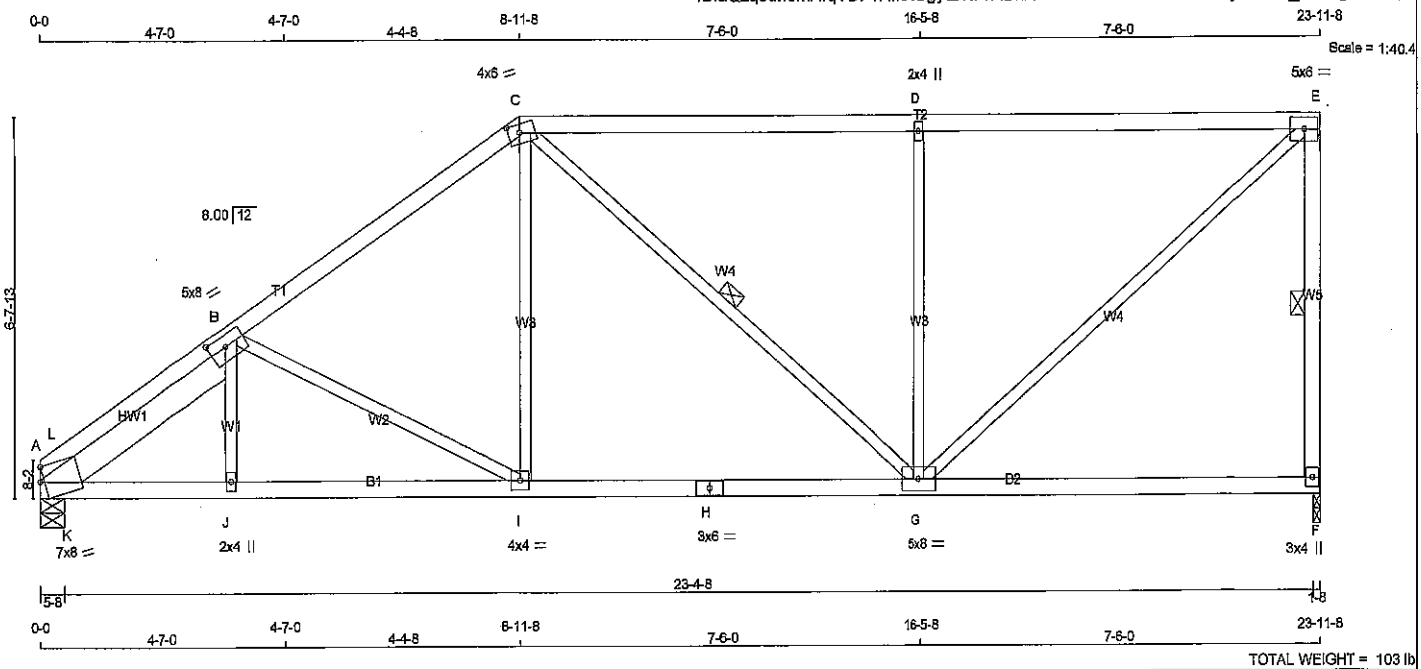


TOTAL WEIGHT = 103 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
A - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
REINFORCING MEMBERS				
HW1	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBMW1-m	MT20	7.0	8.0	3.00	Edge
B	TMWWW-t	MT20	5.0	8.0	2.25	3.50
C	TTWW-m	MT20	4.0	6.0	1.75	2.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	5.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	8.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMW+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
A	1345	0	1345	0	0	5-8	5-8
F	1345	0	1345	0	0	1-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	1104	613 / 0	252 / 0	0 / 0	0 / 0	240 / 0	0 / 0
F	1104	613 / 0	252 / 0	0 / 0	0 / 0	240 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.03 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, C-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- L	-1582 / 0	-84.3	-84.3	0.06 (1)	5.21		J- B	0 / 220	0.05 (2)
L- B	-1097 / 0	-84.3	-84.3	0.31 (1)	5.62		B- I	-426 / 0	0.28 (1)
B- C	-1465 / 0	-84.3	-84.3	0.37 (1)	5.01		I- C	0 / 443	0.10 (2)
C- D	-1209 / 0	-84.3	-84.3	0.94 (1)	4.03		C- G	-30 / 5	0.02 (3)
D- E	-1209 / 0	-84.3	-84.3	0.94 (1)	4.03		G- D	-784 / 0	0.57 (1)
F- E	-1260 / 0	0.0	0.0	0.26 (1)	5.73		G- E	0 / 1581	0.36 (1)
							K- L	0 / 559	0.00 (1)
A- K	0 / 949	-28.0	-28.0	0.19 (1)	10.00		K- B	-854 / 0	0.13 (1)
K- J	0 / 1581	-28.0	-28.0	0.30 (1)	10.00				
J- I	0 / 1579	-28.0	-28.0	0.41 (2)	10.00				
I- H	0 / 1205	-28.0	-28.0	0.54 (2)	10.00				
H- G	0 / 1205	-28.0	-28.0	0.54 (2)	10.00				
G- F	0 / 0	-28.0	-28.0	0.40 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.80")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.80")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.94/1.00 (D-E:1) , BC=0.54/1.00 (G-I:2) , WB=0.57/1.00 (D-G:1) , SSI=0.31/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

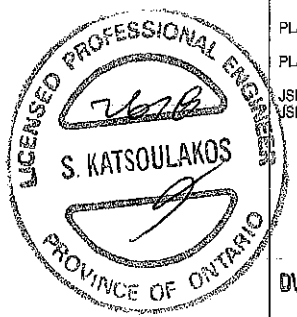
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (G) (INPUT = 0.90)
JSI METAL= 0.47 (H) (INPUT = 1.00)



DWG NO. TAM 6071-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285930	T140X	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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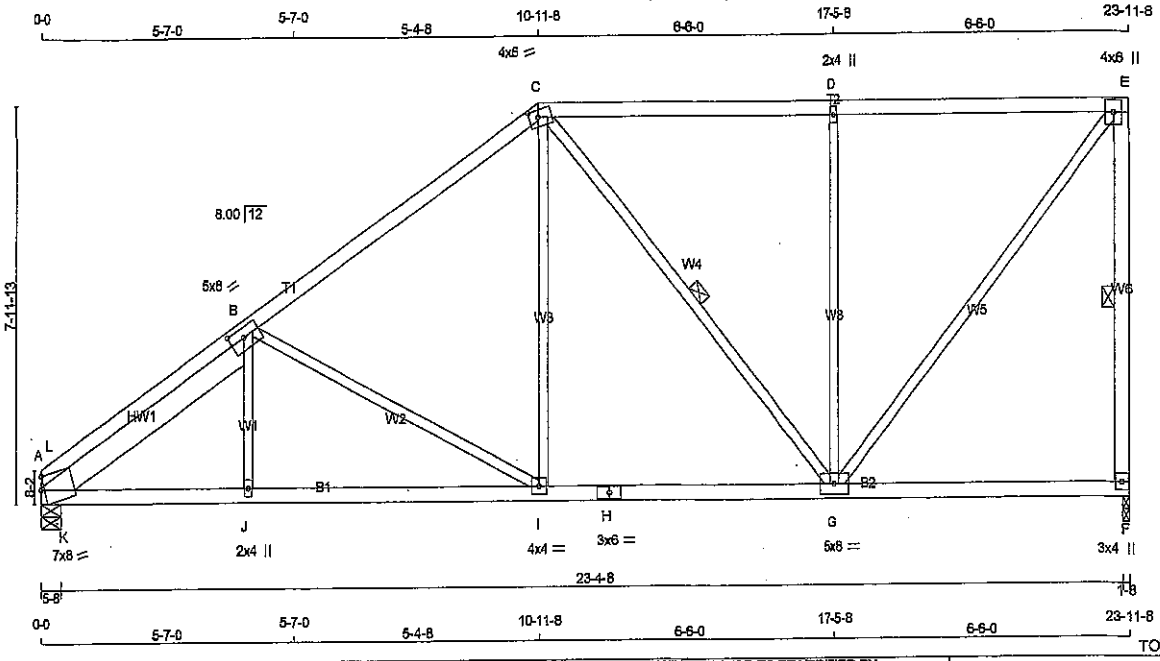

TOTAL WEIGHT = 112 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
A - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
REINFORCING MEMBERS				
HW1	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBMW1-m	MT20	7.0	8.0	3.00	Edge
B	TMWWW-t	MT20	5.0	8.0	2.25	3.50
C	TTWW-m	MT20	4.0	6.0	1.75	2.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW+p	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	6.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMW+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
A	1345	0	1345	0	0	5-8	5-8
F	1345	0	1345	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	1104	613 / 0	252 / 0	0 / 0	0 / 0	240 / 0	0 / 0
F	1104	613 / 0	252 / 0	0 / 0	0 / 0	240 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.03 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, C-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A- L	-1609 / 0	-84.3	-84.3	0.04 (1)	5.20
L- B	-1135 / 0	-84.3	-84.3	0.46 (1)	5.29
B- C	-1289 / 0	-84.3	-84.3	0.54 (1)	5.03
C- D	-899 / 0	-84.3	-84.3	0.65 (1)	5.38
D- E	-899 / 0	-84.3	-84.3	0.65 (1)	5.38
F- E	-1271 / 0	0.0	0.0	0.36 (1)	5.71
A- K	0 / 980	-28.0	-28.0	0.19 (1)	10.00
K- J	0 / 1570	-28.0	-28.0	0.37 (2)	10.00
J- I	0 / 1568	-28.0	-28.0	0.41 (2)	10.00
I- H	0 / 1055	-28.0	-28.0	0.41 (2)	10.00
H- G	0 / 1055	-28.0	-28.0	0.41 (2)	10.00
G- F	0 / 0	-28.0	-28.0	0.29 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- B	0 / 286	0.06 (2)
B- I	-589 / 0	0.52 (1)
I- C	0 / 508	0.11 (2)
C- G	-241 / 0	0.17 (1)
G- D	-679 / 0	0.82 (1)
G- E	0 / 1393	0.31 (1)
K- L	0 / 508	0.00 (1)
K- B	-774 / 0	0.19 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.80")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.80")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.65/1.00 (D-E:1) , BC=0.41/1.00 (G-I:2) , WB=0.82/1.00 (D-G:1) , SSI=0.27/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

USI GRIP= 0.86 (E) (INPUT = 0.90)
USI METAL= 0.32 (A) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
1316
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 3342-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285930	T141X	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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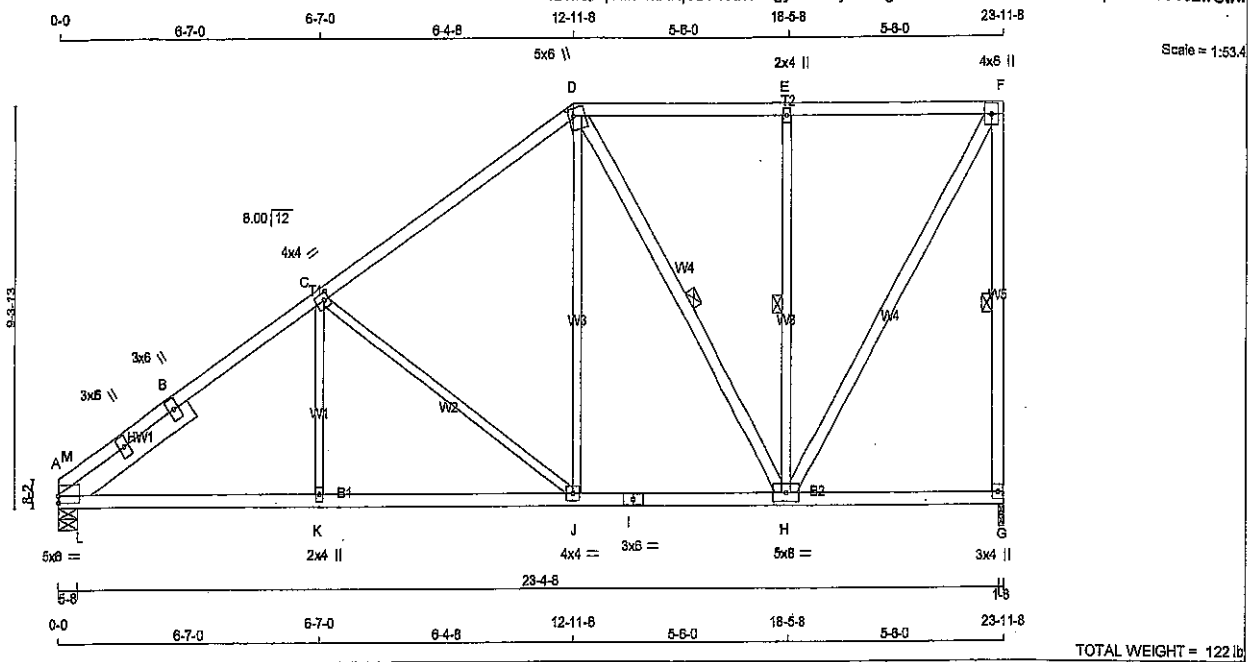

TOTAL WEIGHT = 122 lb

[MiTek]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
A - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
REINFORCING MEMBERS				
HW1	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBMR1-I	MT20	5.0	6.0	2.00	0.25
A	RT+t	MT20	3.0	6.0		
A	RT+t	MT20	3.0	6.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMW+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
A	1345	0	1345	0	0	5-8	5-8
G	1345	0	1345	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	1104	613 / 0	252 / 0	0 / 0	0 / 0	240 / 0	0 / 0
G	1104	613 / 0	252 / 0	0 / 0	0 / 0	240 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- M	-1706 / 0	-84.3	-84.3	0.06 (1)	5.06
M- B	-1480 / 0	-84.3	-84.3	0.40 (1)	4.98
B- C	-1480 / 0	-84.3	-84.3	0.40 (1)	4.98
C- D	-1100 / 0	-84.3	-84.3	0.43 (1)	5.48
D- E	-677 / 0	-84.3	-84.3	0.33 (1)	6.25
E- F	-678 / 0	-84.3	-84.3	0.33 (1)	6.25
G- F	-1281 / 0	0.0	0.0	0.52 (1)	5.89
A- L	0 / 1074	-28.0	-28.0	0.25 (2)	10.00
L- K	0 / 1444	-28.0	-28.0	0.41 (2)	10.00
K- J	0 / 1444	-28.0	-28.0	0.41 (2)	10.00
J- I	0 / 897	-28.0	-28.0	0.30 (2)	10.00
I- H	0 / 897	-28.0	-28.0	0.30 (2)	10.00
H- G	0 / 0	-28.0	-28.0	0.20 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	0 / 326	0.07 (2)
C- J	-687 / 0	0.93 (1)
J- D	0 / 570	0.13 (2)
D- H	-423 / 0	0.25 (1)
H- E	-574 / 0	0.33 (1)
H- F	0 / 1295	0.21 (1)
L- M	0 / 489	0.00 (1)
L- B	-515 / 0	0.06 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.80")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.80")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.52/1.00 (F-G:1) , BC=0.41/1.00 (K-L:2) , WB=0.93/1.00 (C-J:1) , SSI=0.23/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1867	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.72 (F) (INPUT = 0.90)
JSI METAL= 0.49 (A) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
1/11/18
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM3343-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285473	T142	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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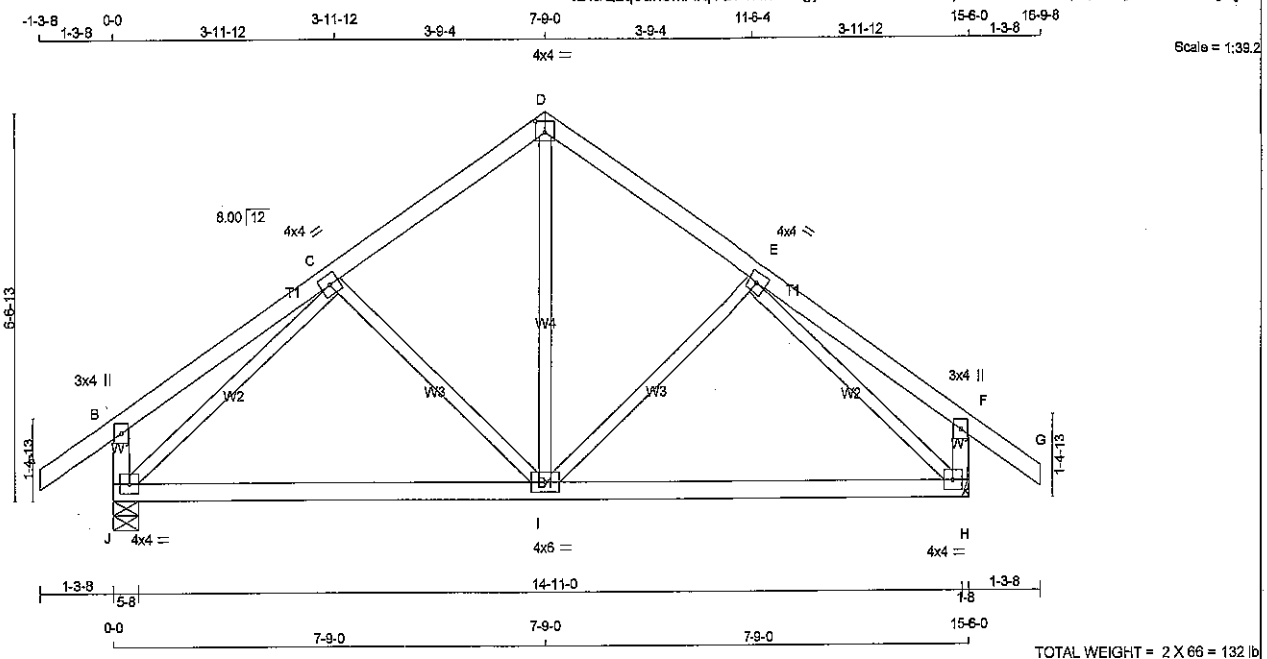


TOTAL WEIGHT = 2 X 66 = 132 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT	REQRD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	BRG
J	986	0	986	0	0	IN-SX 5-8	IN-SX 5-8
H	986	0	986	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	793	467 / 0	163 / 0	0 / 0	0 / 0	163 / 0	0 / 0
H	793	467 / 0	163 / 0	0 / 0	0 / 0	163 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	
B-C	0 / 21	-84.3	-84.3	0.20 (1)	10.00	
C-D	-708 / 0	-84.3	-84.3	0.16 (1)	6.25	
D-E	-708 / 0	-84.3	-84.3	0.16 (1)	6.25	
E-F	0 / 21	-84.3	-84.3	0.20 (1)	10.00	
F-G	0 / 32	-84.3	-84.3	0.11 (1)	10.00	
J-B	-241 / 0	0.0	0.0	0.02 (1)	7.81	
H-F	-241 / 0	0.0	0.0	0.02 (1)	7.81	
J-I	0 / 700	-28.0	-28.0	0.57 (2)	10.00	
I-H	0 / 700	-28.0	-28.0	0.57 (2)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I-D	0 / 509	0.11 (1)
I-E	-175 / 43	0.08 (1)
C-I	-175 / 43	0.08 (1)
J-C	-963 / 0	0.44 (1)
E-H	-963 / 0	0.44 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.52")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.52")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.20 (E-F:1) , BC=0.57 (H-I:2) , WB=0.44 (E-H:1) , SSI=0.18 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

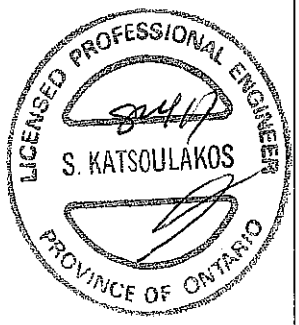
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (D) (INPUT = 0.90)
JSI METAL= 0.34 (C) (INPUT = 1.00)



DWG NO. TAM 41982-17
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285473	G142	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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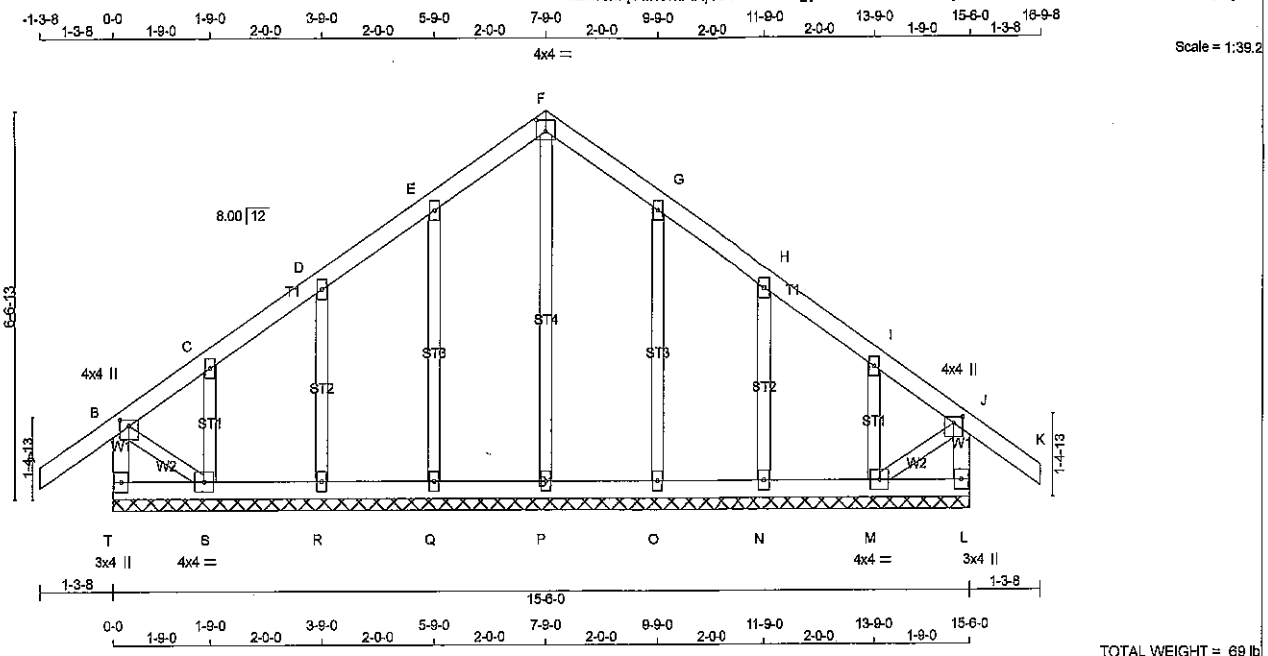

TOTAL WEIGHT = 69 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - F	2x4	DRY	No.2	SPF
F - K	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C, D, E, G, H, I						
C	TMW+w	MT20	2.0	4.0		
F	TTW-p	MT20	4.0	4.0	2.25	2.00
J	TMVW+p	MT20	4.0	4.0	1.25	2.00
L	BMV1+p	MT20	3.0	4.0		
M	BMWW1-t	MT20	4.0	4.0		
N, O, P, Q, R						
N	BMW1+w	MT20	2.0	4.0		
S	BMWW1-t	MT20	4.0	4.0		
T	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
FR-TO					
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00
B- C	-47 / 0	-84.3	-84.3	0.11 (1)	6.25
C- D	-14 / 0	-84.3	-84.3	0.04 (1)	6.25
D- E	-13 / 0	-84.3	-84.3	0.05 (1)	6.25
E- F	-20 / 0	-84.3	-84.3	0.05 (1)	6.25
F- G	-20 / 0	-84.3	-84.3	0.05 (1)	6.25
G- H	-13 / 0	-84.3	-84.3	0.05 (1)	6.25
H- I	-14 / 0	-84.3	-84.3	0.04 (1)	6.25
I- J	-47 / 0	-84.3	-84.3	0.11 (1)	6.25
J- K	0 / 32	-84.3	-84.3	0.11 (1)	10.00
T- B	-252 / 0	0.0	0.0	0.03 (1)	7.81
L- J	-252 / 0	0.0	0.0	0.03 (1)	7.81
T- S	0 / 0	-28.0	-28.0	0.02 (2)	10.00
S- R	0 / 16	-28.0	-28.0	0.02 (2)	10.00
R- Q	0 / 12	-28.0	-28.0	0.02 (2)	10.00
Q- P	0 / 9	-28.0	-28.0	0.02 (2)	10.00
P- O	0 / 9	-28.0	-28.0	0.02 (2)	10.00
O- N	0 / 12	-28.0	-28.0	0.02 (2)	10.00
N- M	0 / 16	-28.0	-28.0	0.02 (2)	10.00
M- L	0 / 0	-28.0	-28.0	0.02 (2)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
P- F	-129 / 0	0.09 (1)
Q- E	-189 / 0	0.08 (1)
R- D	-173 / 0	0.04 (1)
S- C	-102 / 0	0.02 (1)
O- G	-189 / 0	0.08 (1)
N- H	-173 / 0	0.04 (1)
M- I	-102 / 0	0.02 (1)
B- S	0 / 23	0.01 (1)
M- J	0 / 23	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11 (A-B:1) , BC=0.02 (M-N:2) , WB=0.09 (F-P:1) , SSI=0.08 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.16 (J) (INPUT = 0.90)
JSI METAL= 0.05 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEERS
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 41987-17
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285473	T143	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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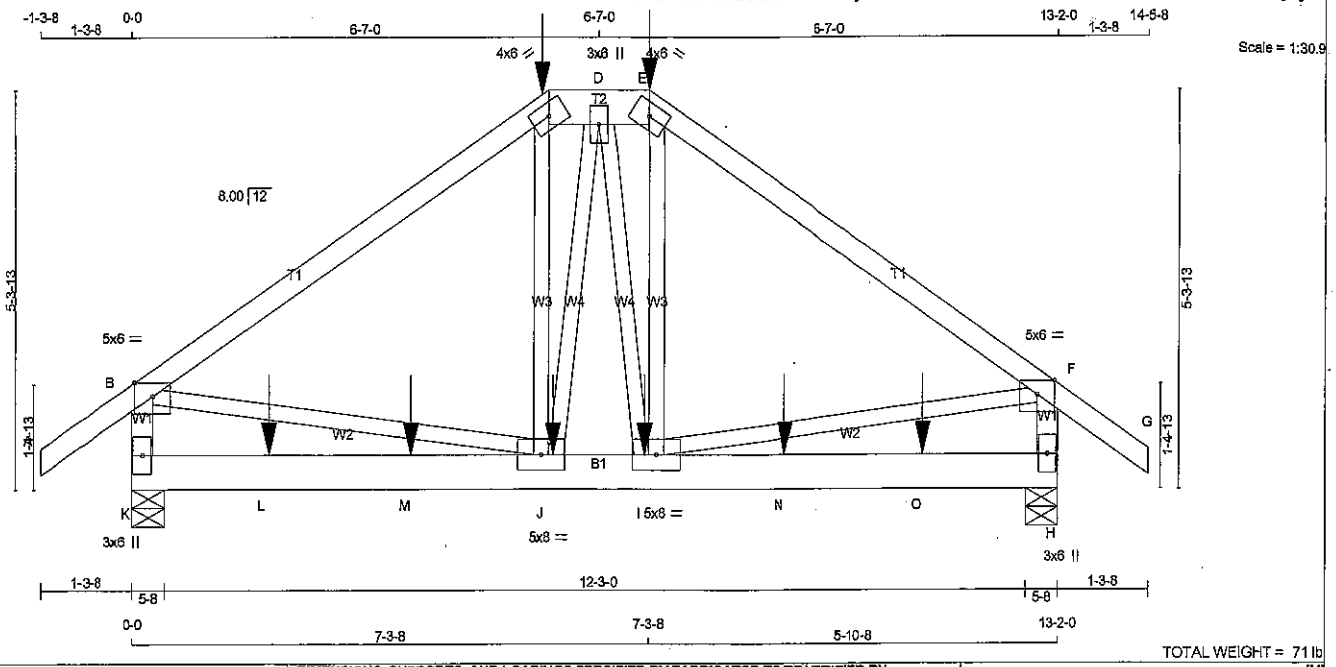


TOTAL WEIGHT = 71 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
K - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TTW-h	MT20	4.0	6.0		
D	TMWW+t	MT20	3.0	6.0		
E	TTW-h	MT20	4.0	6.0		
F	TMVW-p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	6.0		
I	BMWWW-t	MT20	5.0	8.0		
J	BMWWW-t	MT20	5.0	8.0		
K	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 474.0 lbs FACTORED DOWN AT 5-10-8, AND 474.0 lbs FACTORED DOWN AT 7-3-8 ON TOP CHORD, AND 87.4 lbs FACTORED DOWN AT 1-11-4, 87.4 lbs FACTORED DOWN AT 3-11-4, 71.7 lbs FACTORED DOWN AT 5-11-4, 71.7 lbs FACTORED DOWN AT 7-2-12, AND 87.4 lbs FACTORED DOWN AT 9-2-12, AND 87.4 lbs FACTORED DOWN AT 11-2-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	1469	0	1469	0	0	5-8	5-8
H	1469	0	1469	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	1183	693 / 0	244 / 0	0 / 0	0 / 0	247 / 0	0 / 0
H	1183	693 / 0	244 / 0	0 / 0	0 / 0	247 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.51 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX. CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 32	-84.3	-84.3	0.12 (1)	10.00		J- C	0 / 196	0.05 (3)
B- C	-1432 / 0	-84.3	-84.3	0.68 (1)	4.51		I- E	0 / 172	0.04 (3)
C- D	-1192 / 0	-84.3	-84.3	0.07 (2)	6.25		B- J	0 / 1211	0.30 (1)
D- E	-1192 / 0	-84.3	-84.3	0.07 (2)	6.25		I- F	0 / 1211	0.30 (1)
E- F	-1432 / 0	-84.3	-84.3	0.68 (1)	4.51		J- D	-10 / 96	0.02 (3)
F- G	0 / 32	-84.3	-84.3	0.12 (1)	10.00		D- I	-12 / 116	0.03 (3)
K- B	-1356 / 0	0.0	0.0	0.15 (1)	6.93				
H- F	-1356 / 0	0.0	0.0	0.15 (1)	6.93				
K- L	0 / 0	-28.0	-28.0	0.20 (2)	10.00				
L- M	0 / 0	-28.0	-28.0	0.20 (2)	10.00				
M- J	0 / 0	-28.0	-28.0	0.20 (2)	10.00				
J- I	0 / 1195	-28.0	-28.0	0.29 (2)	10.00				
I- N	0 / 0	-28.0	-28.0	0.20 (2)	10.00				
N- O	0 / 0	-28.0	-28.0	0.20 (2)	10.00				
O- H	0 / 0	-28.0	-28.0	0.20 (2)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	5-10-8	-474	-474	---	FRONT	VERT	TOTAL
E	7-3-8	-474	-474	---	FRONT	VERT	TOTAL
I	7-2-12	-41	-72	---	FRONT	VERT	TOTAL
J	5-11-4	-41	-72	---	FRONT	VERT	TOTAL
L	1-11-4	-50	-87	---	FRONT	VERT	TOTAL
M	3-11-4	-50	-87	---	FRONT	VERT	TOTAL
N	9-2-12	-50	-87	---	FRONT	VERT	TOTAL
O	11-2-12	-50	-87	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.44")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.44")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.68 (E-F:1) , BC=0.29 (I-J:2) , WB=0.30 (F-I:1) , SSI=0.16 (J-K:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

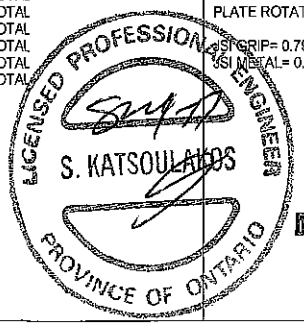
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

LSI GRIP= 0.79 (B) (INPUT = 0.90)
LSI METAL= 0.29 (F) (INPUT = 1.00)



DWG NO. TAM 41983-17
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285473.	T144	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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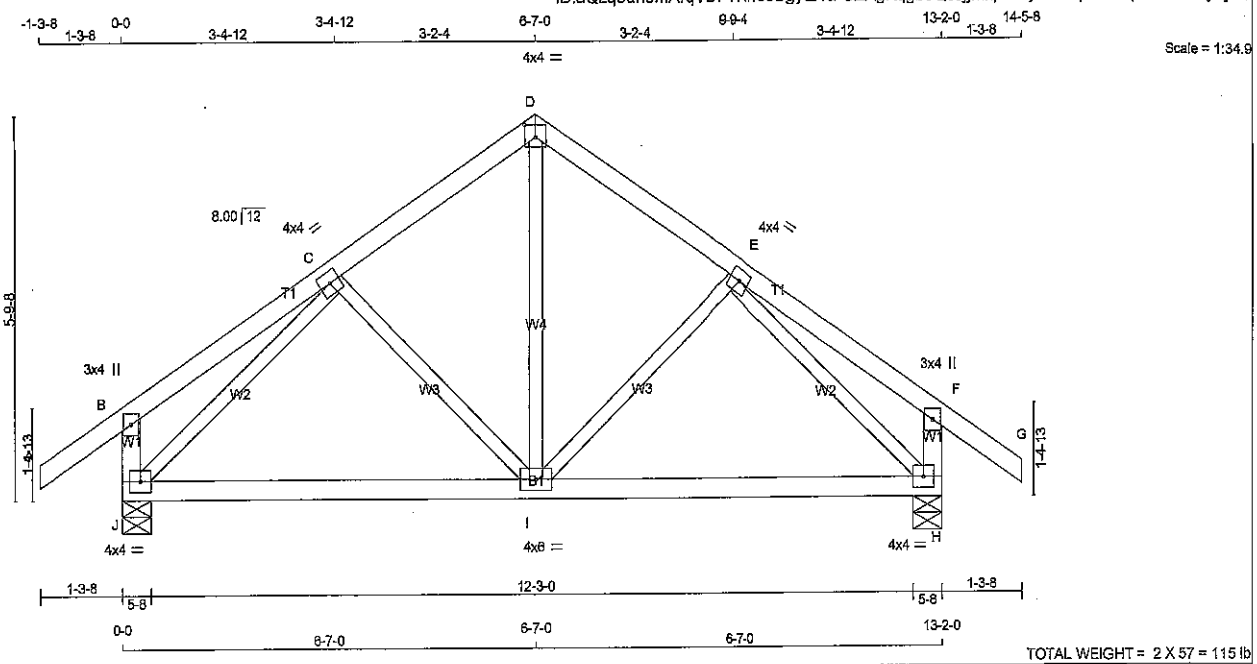


TOTAL WEIGHT = 2 X 57 = 115 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	855	0	855	0	0	5-8	5-8
H	855	0	855	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	685	407 / 0	138 / 0	0 / 0	0 / 0	140 / 0	0 / 0
H	685	407 / 0	138 / 0	0 / 0	0 / 0	140 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	I- D	0 / 411	0.09 (1)	
B- C	0 / 17	-84.3	-84.3	0.14 (1)	10.00	I- E	-130 / 44	0.05 (1)	
C- D	-583 / 0	-84.3	-84.3	0.11 (1)	6.25	C- I	-130 / 44	0.05 (1)	
D- E	-583 / 0	-84.3	-84.3	0.11 (1)	6.25	J- C	-795 / 0	0.27 (1)	
E- F	0 / 17	-84.3	-84.3	0.14 (1)	10.00	E- H	-795 / 0	0.27 (1)	
F- G	0 / 32	-84.3	-84.3	0.11 (1)	10.00				
J- B	-223 / 0	0.0	0.0	0.02 (1)	7.81				
H- F	-223 / 0	0.0	0.0	0.02 (1)	7.81				
J- I	0 / 563	-28.0	-28.0	0.41 (2)	10.00				
I- H	0 / 563	-28.0	-28.0	0.41 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.44")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.44")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.14 (E-F:1) , BC=0.41 (I-J:2) , WB=0.27 (E-H:1) , SSI=0.16 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

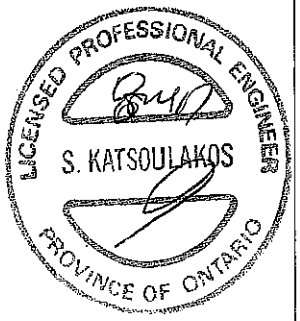
NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.70 (C) (INPUT = 0.90)
JSI METAL= 0.28 (C) (INPUT = 1.00)



DWG NO. TAM41964-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285473	P20	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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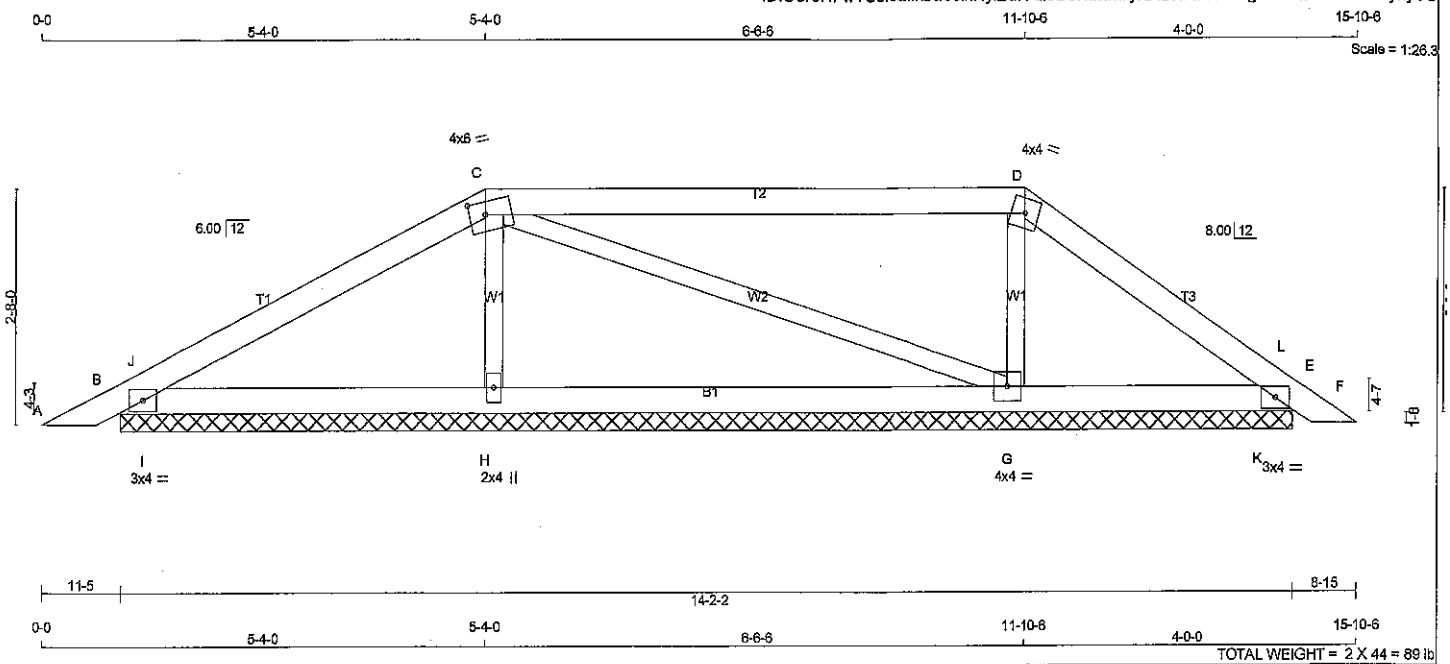


TOTAL WEIGHT = 2 X 44 = 89 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW-m	MT20	4.0	6.0	1.75	2.25
D	TTW-m	MT20	4.0	4.0		
E	TMB1-I	MT20	3.0	4.0		
G	BMWW1-t	MT20	4.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
B	349	0	349	0	0	14-2-2	14-2-2
E	266	0	266	0	0	14-2-2	14-2-2
H	581	0	581	0	0	14-2-2	14-2-2
G	526	0	526	0	0	14-2-2	14-2-2

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	263	184 / 0	35 / 0	0 / 0	0 / 0	45 / 0	0 / 0
E	195	148 / 0	19 / 0	0 / 0	0 / 0	30 / 0	0 / 0
H	490	250 / 0	126 / 0	0 / 0	0 / 0	114 / 0	0 / 0
G	446	224 / 0	118 / 0	0 / 0	0 / 0	105 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS					WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 15	-84.3	-64.3	0.05 (1)	10.00	H- C	-374 / 0	0.06 (1)
B- J	-65 / 0	-84.3	-84.3	0.08 (1)	6.25	C- G	-27 / 0	0.02 (1)
J- C	-116 / 0	-84.3	-84.3	0.20 (1)	6.25	G- D	-341 / 0	0.05 (1)
C- D	-56 / 0	-84.3	-84.3	0.61 (1)	6.25	I- J	-286 / 39	0.00 (1)
D- L	-93 / 0	-84.3	-84.3	0.11 (1)	6.25	K- L	-232 / 0	0.00 (1)
L- E	-32 / 0	-84.3	-84.3	0.04 (1)	6.25			
E- F	0 / 14	-84.3	-84.3	0.03 (1)	10.00			
B- I	0 / 100	-28.0	-28.0	0.18 (1)	10.00			
I- H	0 / 100	-28.0	-28.0	0.22 (2)	10.00			
H- G	0 / 82	-28.0	-28.0	0.22 (2)	10.00			
G- K	0 / 72	-28.0	-28.0	0.19 (2)	10.00			
K- E	0 / 72	-28.0	-28.0	0.11 (1)	10.00			

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.61 (C-D:1) , BC=0.22 (H-I:2) , WB=0.06 (C-H:1) , SSI=0.23 (B-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

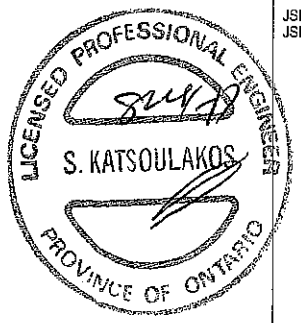
NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.26 (B) (INPUT = 0.90)
JSI METAL= 0.07 (H) (INPUT = 1.00)



DWG NO. TAM 41988-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285473	P21	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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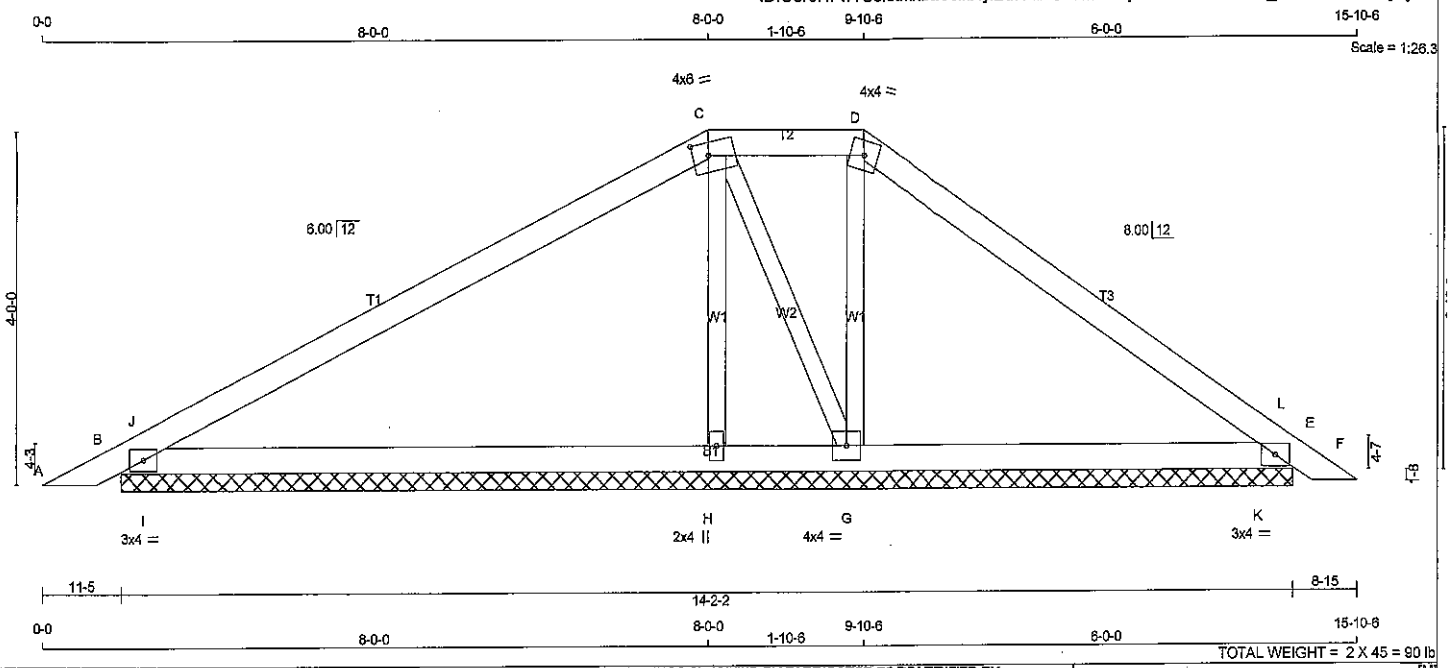

TOTAL WEIGHT = 2 X 45 = 90 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW-m	MT20	4.0	6.0	1.75	2.25
D	TTW-m	MT20	4.0	4.0		
E	TMB1-I	MT20	3.0	4.0		
G	BMWW1-t	MT20	4.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	547	0	547	0	0	14-2-2	14-2-2
E	449	0	449	0	0	14-2-2	14-2-2
H	413	0	413	0	0	14-2-2	14-2-2
G	312	0	312	0	0	14-2-2	14-2-2

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	429	271 / 0	76 / 0	0 / 0	0 / 0	82 / 0	0 / 0
E	355	219 / 0	66 / 0	0 / 0	0 / 0	70 / 0	0 / 0
H	372	153 / 0	121 / 0	0 / 0	0 / 0	99 / 0	0 / 0
G	238	161 / 0	35 / 0	0 / 0	0 / 0	42 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 15	-84.3	-84.3	0.05 (1)	10.00	H- C	-131 / 4	0.03 (1)
B- J	-196 / 0	-84.3	-84.3	0.21 (1)	6.25	C- G	-107 / 0	0.03 (1)
J- C	-253 / 0	-84.3	-84.3	0.55 (1)	6.25	G- D	-157 / 11	0.04 (1)
C- D	-166 / 0	-84.3	-84.3	0.05 (1)	6.25	I- J	-872 / 205	0.00 (1)
D- L	-215 / 0	-84.3	-84.3	0.30 (1)	6.25	K- L	-480 / 170	0.00 (1)
L- E	-199 / 0	-84.3	-84.3	0.11 (1)	6.25			
E- F	0 / 14	-84.3	-84.3	0.03 (1)	10.00			
B- I	0 / 217	-28.0	-28.0	0.42 (1)	10.00			
I- H	0 / 217	-28.0	-28.0	0.42 (1)	10.00			
H- G	0 / 213	-28.0	-28.0	0.37 (1)	10.00			
G- K	0 / 171	-28.0	-28.0	0.25 (1)	10.00			
K- E	0 / 171	-28.0	-28.0	0.24 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.55 (C-J:1) , BC=0.42 (H-I:1) , WB=0.04 (D-G:1) , SSI=0.55 (B-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	922	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.43 (B) (INPUT = 0.90)
JSI METAL= 0.12 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 41969-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285473	P22	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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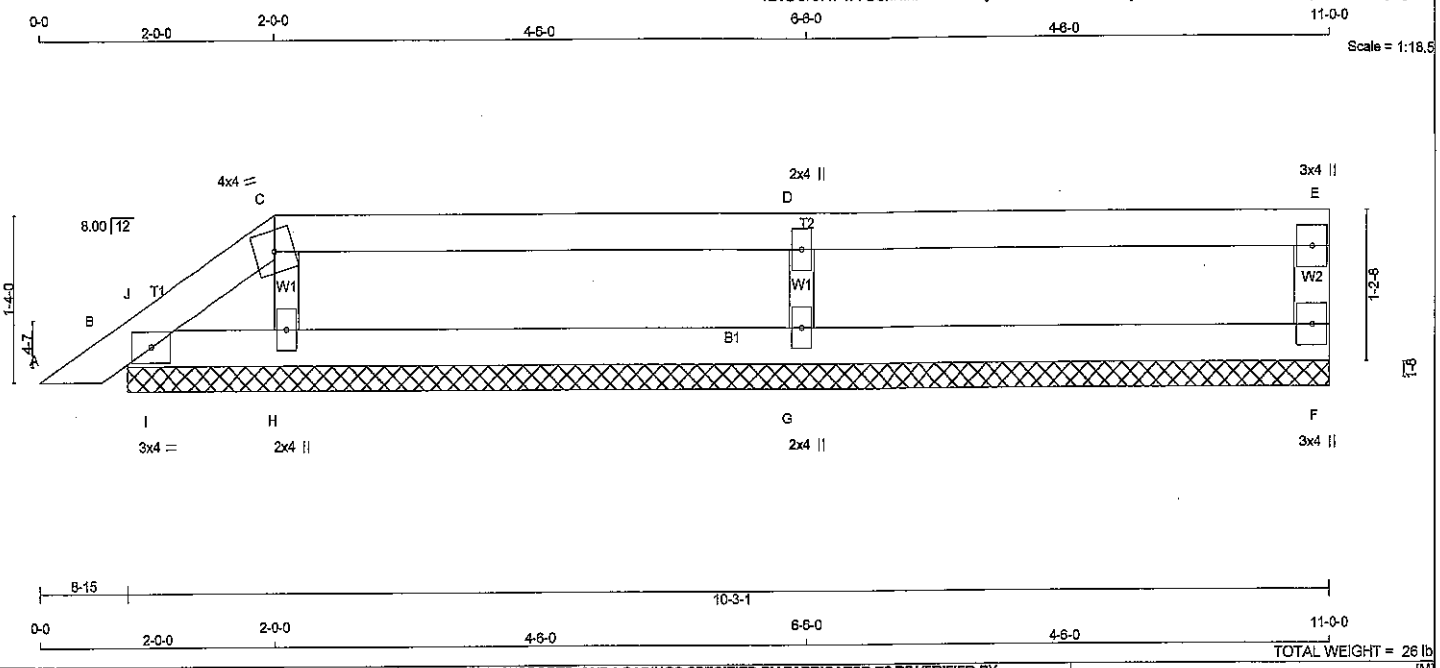

TOTAL WEIGHT = 26 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW-m	MT20	4.0	4.0		
D	TMW+w	MT20	2.0	4.0		
E	TMV+p	MT20	3.0	4.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMW1+w	MT20	2.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	194	0	194	0	0	10-3-1	10-3-1
B	120	0	120	0	0	10-3-1	10-3-1
H	274	0	274	0	0	10-3-1	10-3-1
G	619	0	619	0	0	10-3-1	10-3-1

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	160	68 / 0	37 / 0	0 / 0	0 / 0	35 / 0	0 / 0
B	80	74 / 0	0 / -2	0 / 0	0 / 0	8 / 0	0 / 0
H	239	109 / 0	70 / 0	0 / 0	0 / 0	60 / 0	0 / 0
G	503	287 / 0	109 / 0	0 / 0	0 / 0	107 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 14	-84.3	-84.3	0.03 (1)	10.00	H- C	-164 / 0	0.02 (1)
B- J	-15 / 15	-84.3	-84.3	0.01 (3)	6.25	G- D	-474 / 0	0.07 (1)
J- C	-25 / 0	-84.3	-84.3	0.01 (2)	6.25	I- J	-70 / 0	0.00 (1)
C- D	0 / 0	-84.3	-84.3	0.29 (1)	10.00			
D- E	0 / 0	-84.3	-84.3	0.29 (1)	10.00			
F- E	-144 / 0	0.0	0.0	0.01 (1)	7.81			
B- I	0 / 19	-28.0	-28.0	0.02 (1)	10.00			
I- H	0 / 19	-28.0	-28.0	0.07 (2)	10.00			
H- G	0 / 0	-28.0	-28.0	0.15 (3)	10.00			
G- F	0 / 0	-28.0	-28.0	0.15 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.29 (C-D:1) , BC=0.15 (G-H:3) , WB=0.07 (D-G:1) , SSI=0.19 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (G) (INPUT = 0.90)
JSI METAL= 0.09 (G) (INPUT = 1.00)

DWG NO. TAM41990-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285473	P23	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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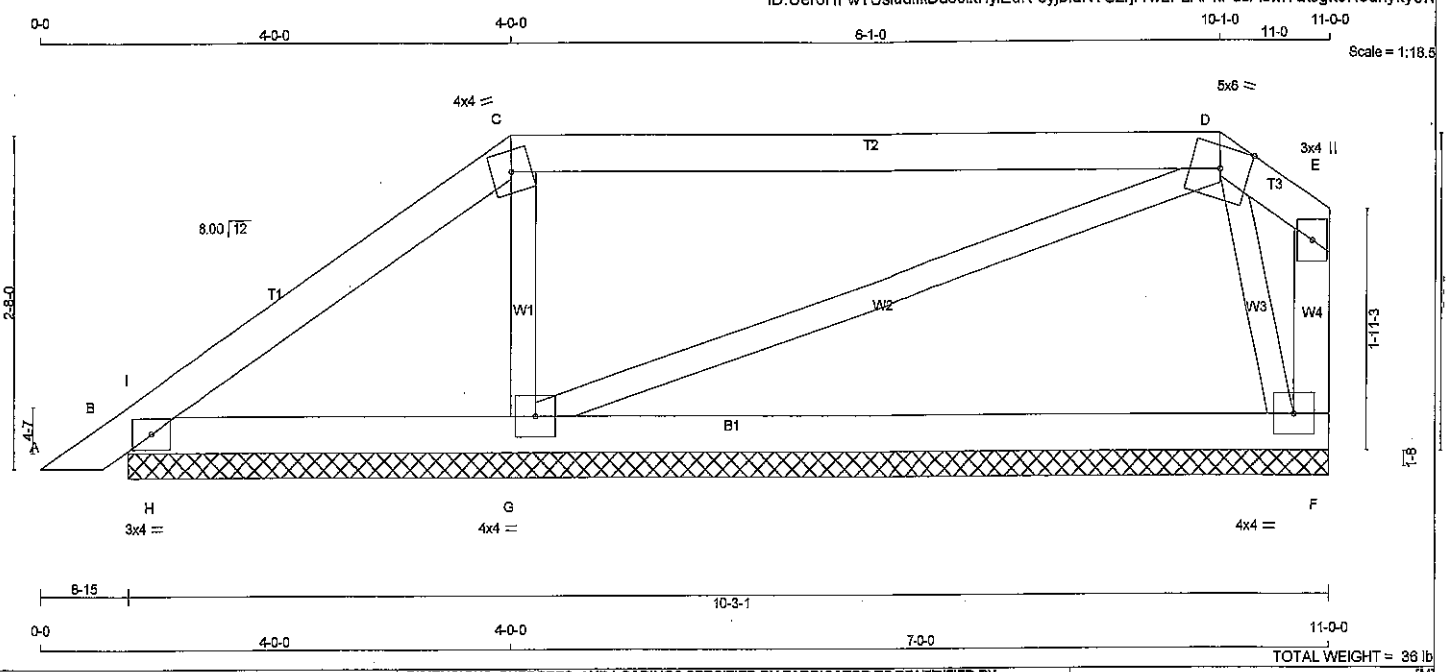

TOTAL WEIGHT = 36 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW-m	MT20	4.0	4.0		
D	TTWW-m	MT20	5.0	6.0	Edge	
E	TMV+p	MT20	3.0	4.0		
F	BMVW1-t	MT20	4.0	4.0		
G	BMWW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	390	0	390	0	0	10-3-1	10-3-1
B	250	0	250	0	0	10-3-1	10-3-1
G	568	0	568	0	0	10-3-1	10-3-1

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	308	190 / 0	58 / 0	0 / 0	0 / 0	61 / 0	0 / 0
B	173	148 / 0	5 / 0	0 / 0	0 / 0	21 / 0	0 / 0
G	502	221 / 0	153 / 0	0 / 0	0 / 0	128 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 14	-84.3	-84.3	3	0.03 (1)	10.00	G- C	-317 / 0	0.05 (1)
B- I	-3 / 50	-84.3	-84.3	3	0.06 (1)	10.00	G- D	-58 / 0	0.04 (1)
I- C	-96 / 0	-84.3	-84.3	3	0.10 (1)	6.25	D- F	-297 / 0	0.05 (1)
C- D	-58 / 2	-84.3	-84.3	3	0.53 (1)	6.25	H- I	-284 / 0	0.00 (1)
D- E	0 / 0	-84.3	-84.3	3	0.01 (1)	10.00			
F- E	-39 / 0	0.0	0.0	3	0.00 (1)	7.81			
B- H	0 / 73	-28.0	-28.0	3	0.12 (1)	10.00			
H- G	0 / 73	-28.0	-28.0	3	0.31 (2)	10.00			
G- F	0 / 113	-28.0	-28.0	3	0.32 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.53 (C-D:1) , BC=0.32 (F-G:2) , WB=0.05 (C-G:1) , SSI=0.21 (B-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.38 (D) (INPUT = 0.90)
JSI METAL= 0.09 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 4199-17
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285473	P24	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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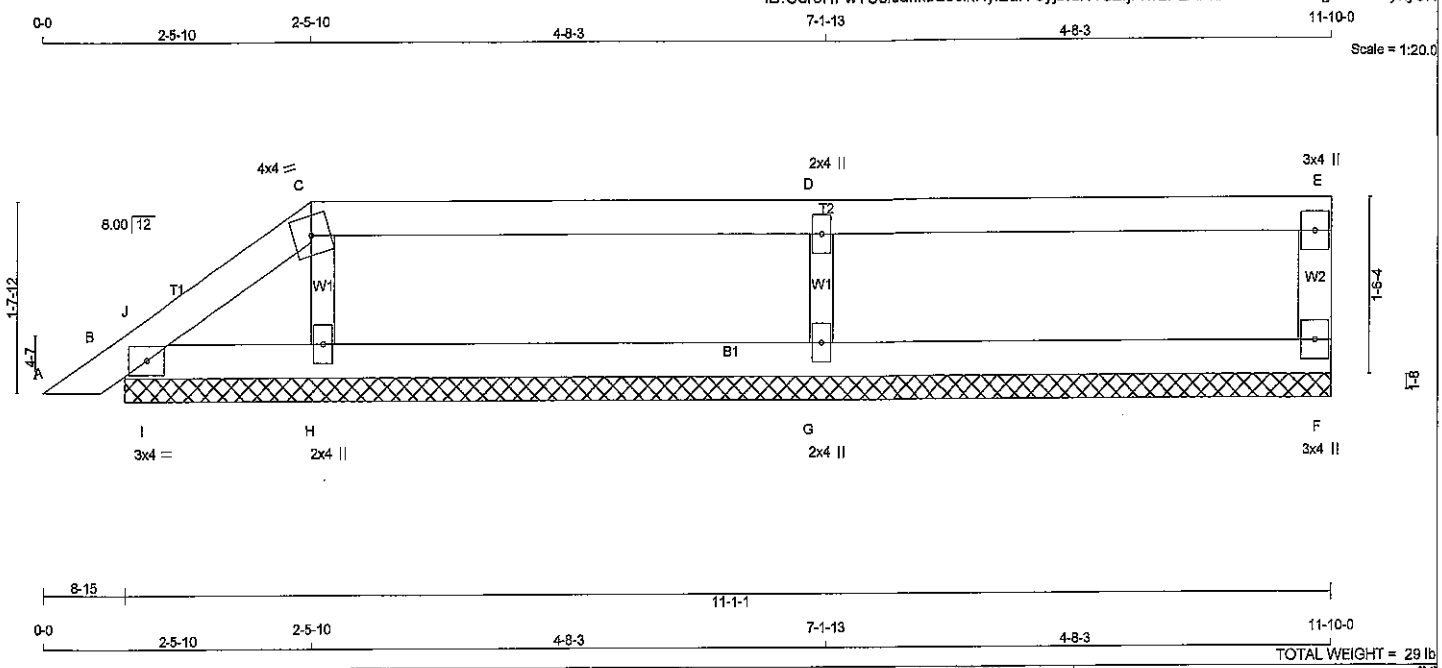

TOTAL WEIGHT = 29 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-i	MT20	3.0	4.0		
C	TTW-m	MT20	4.0	4.0		
D	TMW+w	MT20	2.0	4.0		
E	TMV+p	MT20	3.0	4.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMW1+w	MT20	2.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	202	0	202	0	0	11-1-1	11-1-1
B	148	0	148	0	0	11-1-1	11-1-1
H	307	0	307	0	0	11-1-1	11-1-1
G	644	0	644	0	0	11-1-1	11-1-1

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	167	91 / 0	39 / 0	0 / 0	0 / 0	37 / 0	0 / 0
B	105	64 / 0	7 / 0	0 / 0	0 / 0	14 / 0	0 / 0
H	264	127 / 0	73 / 0	0 / 0	0 / 0	64 / 0	0 / 0
G	524	299 / 0	114 / 0	0 / 0	0 / 0	111 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, H, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 14	-84.3	-84.3	0.03 (1)	10.00	H- C	-192 / 0	0.03 (1)
B- J	-11 / 7	-84.3	-84.3	0.01 (2)	6.25	G- D	-493 / 0	0.07 (1)
J- C	-22 / 0	-84.3	-84.3	0.03 (1)	6.25	I- J	-90 / 0	0.00 (1)
C- D	0 / 0	-84.3	-84.3	0.31 (1)	10.00			
D- E	0 / 0	-84.3	-84.3	0.31 (1)	10.00			
F- E	-150 / 0	0.0	0.0	0.02 (1)	7.81			
B- I	0 / 16	-28.0	-28.0	0.03 (1)	10.00			
I- H	0 / 16	-28.0	-28.0	0.08 (2)	10.00			
H- G	0 / 0	-28.0	-28.0	0.16 (3)	10.00			
G- F	0 / 0	-28.0	-28.0	0.16 (3)	10.00			

(M)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.31 (C-D:1) , BC=0.16 (G-H:3) , WB=0.07 (D-G:1) , SSI=0.19 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1856

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (G) (INPUT = 0.90)
JSI METAL= 0.09 (D) (INPUT = 1.00)

DWG NO. TAM 41992-17
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285473	P25	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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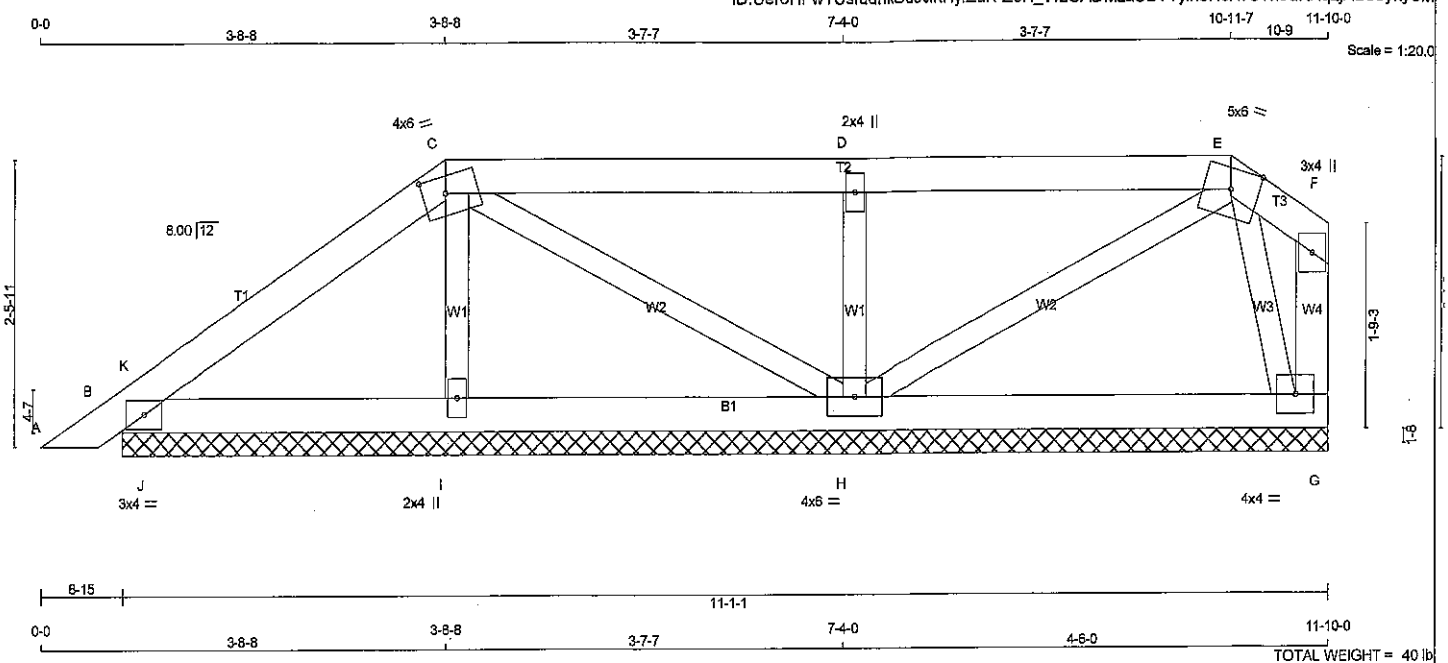

TOTAL WEIGHT = 40 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
B - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW-m	MT20	4.0	6.0	1.75	2.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW-m	MT20	5.0	6.0	Edge	
F	TMV+p	MT20	3.0	4.0		
G	BMWW1-t	MT20	4.0	4.0		
H	BMWWW1-t	MT20	4.0	6.0		
I	BMW1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	209	0	209	0	0	11-1-1	11-1-1
B	251	0	251	0	0	11-1-1	11-1-1
I	286	0	286	0	0	11-1-1	11-1-1
H	554	0	554	0	0	11-1-1	11-1-1

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	170	98 / 0	38 / 0	0 / 0	0 / 0	36 / 0	0 / 0
B	192	129 / 0	29 / 0	0 / 0	0 / 0	34 / 0	0 / 0
I	245	120 / 0	67 / 0	0 / 0	0 / 0	58 / 0	0 / 0
H	452	256 / 0	100 / 0	0 / 0	0 / 0	97 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, B, I, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 14	-84.3	-84.3	0.03 (1)	10.00	I- C	-167 / 0	0.03 (1)
B- K	-55 / 0	-84.3	-84.3	0.02 (3)	6.25	C- H	-45 / 0	0.01 (1)
K- C	-72 / 0	-84.3	-84.3	0.09 (1)	6.25	H- D	-381 / 0	0.06 (1)
C- D	-9 / 0	-84.3	-84.3	0.19 (1)	10.00	H- E	-50 / 0	0.01 (1)
D- E	-9 / 0	-84.3	-84.3	0.19 (1)	10.00	E- G	-132 / 0	0.02 (1)
E- F	0 / 0	-84.3	-84.3	0.01 (1)	10.00	J- K	-168 / 50	0.00 (1)
G- F	-37 / 0	0.0	0.0	0.00 (1)	7.81			
B- J	0 / 57	-28.0	-28.0	0.08 (1)	10.00			
J- I	0 / 57	-28.0	-28.0	0.08 (1)	10.00			
I- H	0 / 49	-28.0	-28.0	0.13 (2)	10.00			
H- G	0 / 52	-28.0	-28.0	0.13 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.19 (C-D:1) , BC=0.13 (H-I:2) , WB=0.06 (D-H:1) , SSI=0.15 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (D) (INPUT = 0.90)
JSI METAL= 0.07 (D) (INPUT = 1.00)

DWG NO. TAM 41993-17
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
285473	P26	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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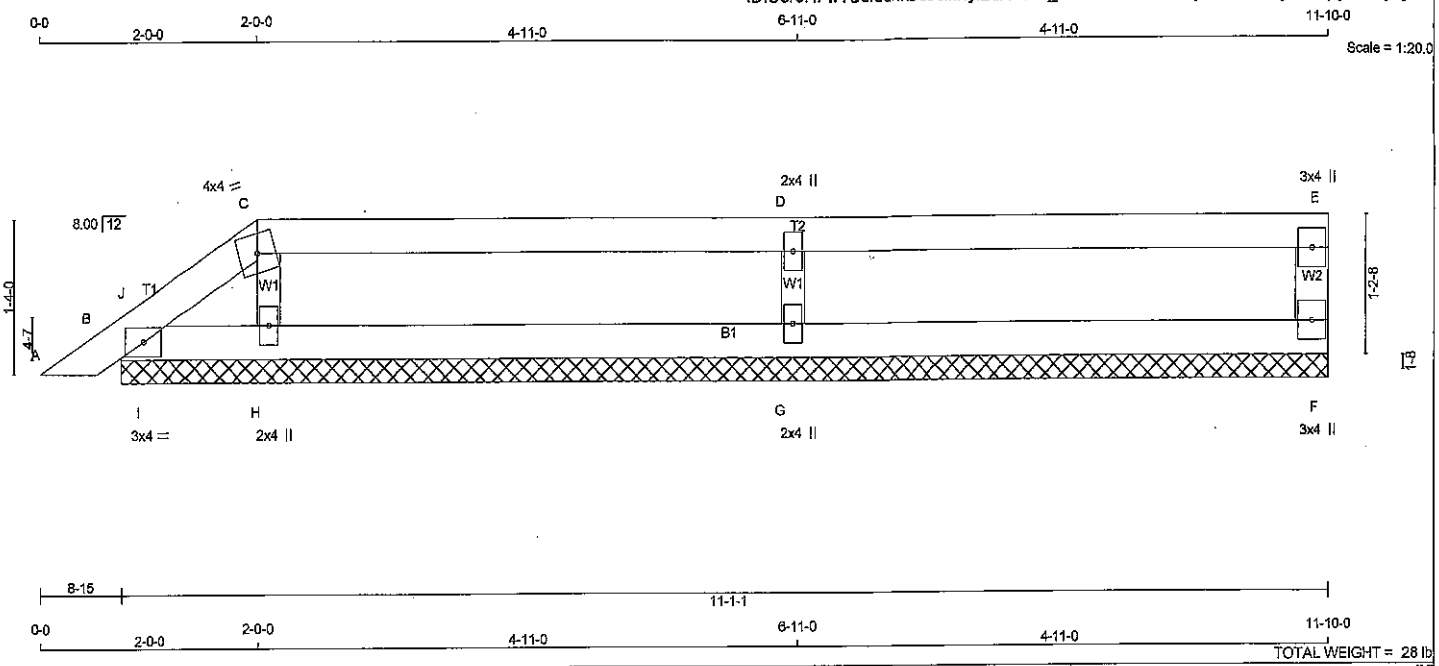


TOTAL WEIGHT = 28 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW-m	MT20	4.0	4.0		
D	TMW+w	MT20	2.0	4.0		
E	TMV+p	MT20	3.0	4.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMW1+w	MT20	2.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	212	0	212	0	0	11-1-1	11-1-1
B	116	0	116	0	-1	11-1-1	11-1-1
H	296	0	296	0	0	11-1-1	11-1-1
G	676	0	676	0	0	11-1-1	11-1-1

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	175	98 / 0	41 / 0	0 / 0	0 / 0	38 / 0	0 / 0
B	75	75 / 0	0 / -5	0 / 0	0 / 0	5 / 0	0 / 0
H	260	117 / 0	78 / 0	0 / 0	0 / 0	66 / 0	0 / 0
G	550	314 / 0	119 / 0	0 / 0	0 / 0	116 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, H, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 14	-84.3	-84.3	0.03 (1)	10.00
B- J	-13 / 22	-84.3	-84.3	0.02 (3)	6.25
J- C	-28 / 0	-84.3	-84.3	0.01 (2)	6.25
C- D	0 / 0	-84.3	-84.3	0.35 (1)	10.00
D- E	0 / 0	-84.3	-84.3	0.35 (1)	10.00
F- E	-158 / 0	0.0	0.0	0.02 (1)	7.81
B- I	0 / 20	-28.0	-28.0	0.02 (1)	10.00
I- H	0 / 20	-28.0	-28.0	0.09 (2)	10.00
H- G	0 / 0	-28.0	-28.0	0.18 (3)	10.00
G- F	0 / 0	-28.0	-28.0	0.18 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-174 / 0	0.02 (1)
G- D	-518 / 0	0.07 (1)
I- J	-77 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.35 (C-D:1) , BC=0.18 (F-G:3) , WB=0.07 (D-G:1) , SSI=0.20 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (G) (INPUT = 0.90)
JSI METAL= 0.10 (D) (INPUT = 1.00)

DWG NO. TAM 41994-17
STRUCTURAL
COMPONENT ONLY

TECHNICAL BULLETIN

LUS – Double Shear Joist Hangers

SIMPSON Strong-Tie

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

MATERIAL: 18 gauge

FINISH: G90 galvanized

DESIGN:

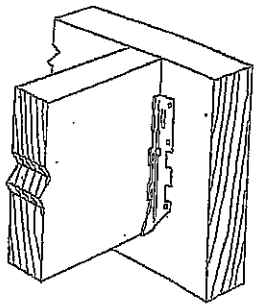
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

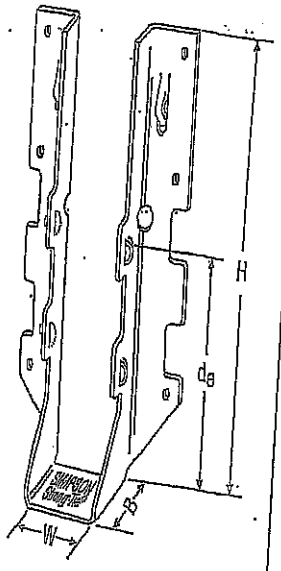
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

- These hangers cannot be modified.



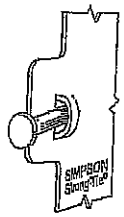
Typical LUS Installation



LUS28

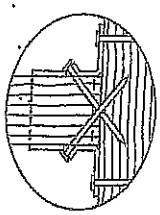
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
								D.Fir-L		S-P-F	
		W	H	B	d_g[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LUS24	18	1⅜	3⅛	1¾	1⅝	4-10d	2-10d	710	1620	945	1155
LUS24-2	18	3⅛	3⅛	2	1⅜	4-16d	2-16d	835	2020	590	1495
LUS26	18	1⅜	4¾	1¾	3⅝	4-10d	4-10d	1420	2170	1290	1630
LUS26-2	18	3⅛	4⅞	2	4	4-16d	4-16d	1720	2595	1545	1920
LUS26-3	18	4⅝	4⅞	2	3¼	4-16d	4-16d	1720	2595	1545	2340
LUS28	18	1⅜	6⅝	1¾	5¾	6-10d	4-10d	1420	2520	1290	1790
LUS28-2	18	3⅛	7	2	4	6-16d	4-16d	1720	3325	1545	2575
LUS28-3	18	4⅝	6¼	2	3¼	6-16d	4-16d	1720	3325	1545	2875
LUS210	18	1⅜	7⅞	1¾	5⅞	8-10d	4-10d	1420	2785	1290	2210
LUS210-2	18	3⅛	9	2	6	8-16d	6-16d	2560	4500	2320	3195
LUS210-3	18	4⅝	8⅜	2	5¼	8-16d	6-16d	2560	3345	2320	2375

1. d_g is the distance from the seat of the hanger to the highest joist nail.

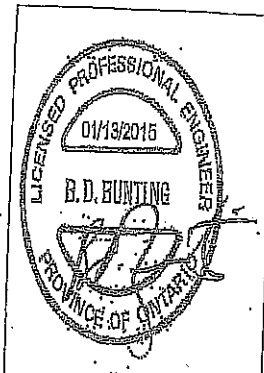


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Top View.



800-999-5099
www.strongtie.com

TECHNICAL BULLETIN

HUS/LJS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: See table

FINISH: G90 galvanized

DESIGN:

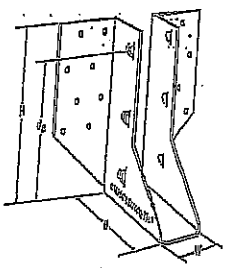
- Factored resistances are in accordance with CSA 086-14
- Uplift resistances have been increased 15% No further increase is permitted
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

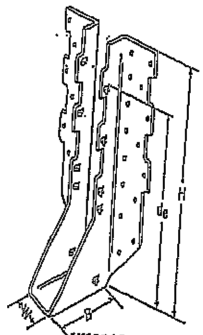
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

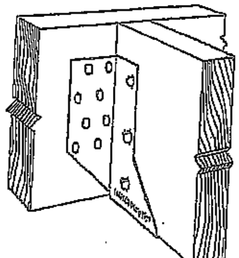
- See current catalogue for options

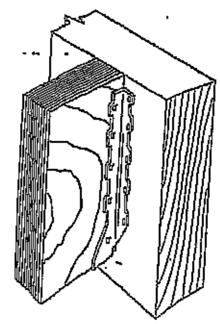


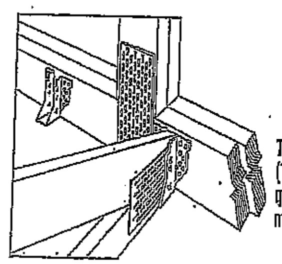
LJS26DS



HUS210
(HUS26, HUS28, similar)

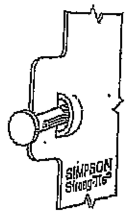

Typical LJS26DS Installation


Typical HUS Installation


Typical HUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)

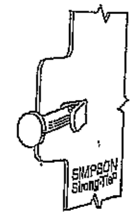
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LJS26DS	18	1 9/16	5	3½	4⅝	16-16d	6-16d	2055	4265	1460	4115
HUS26	16	1⅝	5⅜	3	3 15/16	14-16d	6-16d	2705	4940	2065	3875
HUS28	16	1⅝	7 3/32	3	6 3/32	22-16d	8-16d	3605	5365	2675	4345
HUS210	16	1⅝	9 3/32	3	7 31/32	30-16d	10-16d	4505	5795	4010	4740
HUS1.81/10	16	1 13/16	9	3	8	30-16d	10-16d	4505	6450	4010	5200

1. d_e is the distance from the seat of the hanger to the highest joist nail.

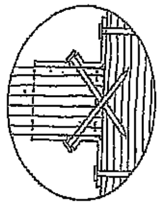


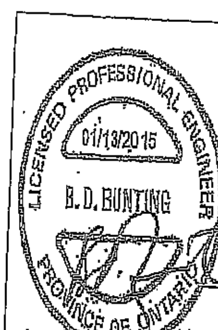
Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.


Double Shear Nailing Top View.



TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

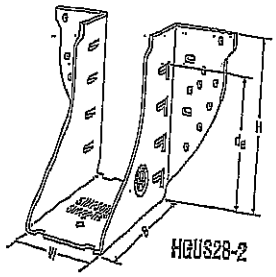
All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: 12 gauge

FINISH: G90 galvanized

DESIGN:

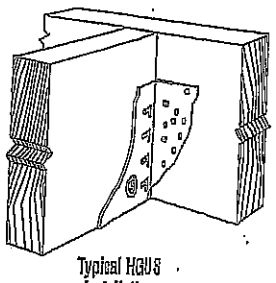
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.



HGUS28-2

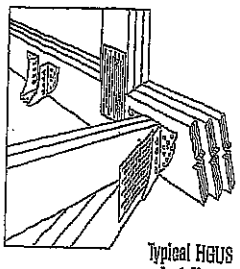
INSTALLATION:

- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications


Typical HGUS Installation

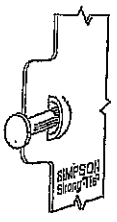
OPTIONS:

- See current catalogue for options.


Typical HGUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)

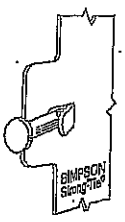
Model No.	Qa	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
								D.Fir-L		S-P-F	
		W	H	B	d_g[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
HGUS26	12	1⅝	5⅜	5	4⅝	20-16d	8-16d	2685	6625	2685	5700
HGUS26-2	12	3⅝	5⅞	4	4⅛	20-16d	8-16d	4385	8950	3100	6355
HGUS26-3	12	4⅝	5½	4	4⅛	20-16d	8-16d	4385	8950	3100	6355
HGUS26-4	12	6⅝	5⅞	4	4⅛	20-16d	8-16d	4385	8950	3100	6355
HGUS28	12	1⅝	7⅛	5	6½	36-16d	12-16d	3310	7675	3100	8900
HGUS28-2	12	3⅝	7⅜	4	6⅛	36-16d	12-16d	6070	12980	4310	9215
HGUS28-3	12	4⅝	7¼	4	6⅜	36-16d	12-16d	6070	12980	4310	9215
HGUS28-4	12	6⅝	7⅜	4	6⅛	36-16d	12-16d	6070	12980	4310	9215
HGU210-2	12	3⅝	9⅜	4	8⅛	46-16d	16-16d	6840	14645	4856	10400
HGUS210-3	12	4⅝	9¼	4	8⅜	46-16d	16-16d	6840	14645	4856	10400
HGUS210-4	12	6⅝	9⅜	4	8⅛	46-16d	16-16d	6840	14645	4856	10400
HGUS212-4	12	6⅝	10⅜	4	10⅛	56-16d	20-16d	7640	14995	5425	10645
HGUS214-4	12	6⅝	12⅜	4	11⅛	66-16d	22-16d	10190	16400	7196	11645

1. d_g is the distance from the seat of the hanger to the highest joist nail.

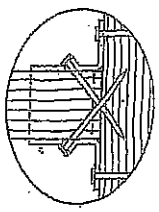


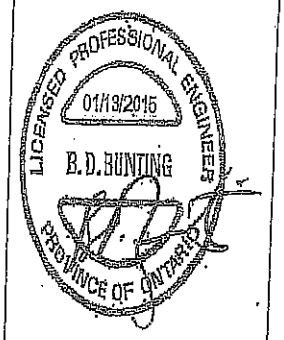
Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.


Double Shear Nailing Top View.



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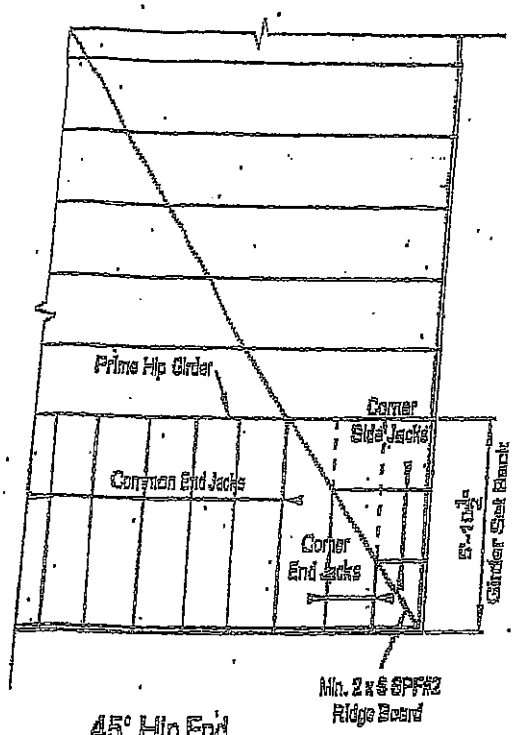


45° Hip End

LUMBER SPECIFICATION

TOP CHORD : 2x4 SPF#2

BOTTOM CHORD : 2x4 SPF#2

WEBS : 2x3 SPF#2

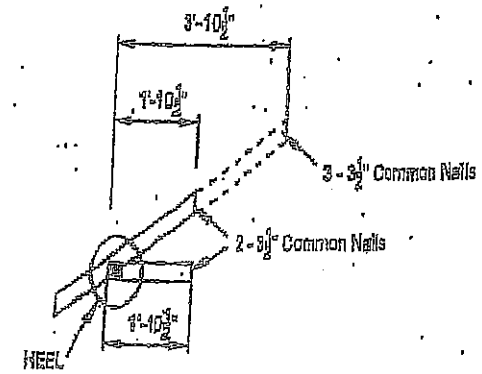
UNLESS OTHERWISE SHOWN

DESIGN LOAD:

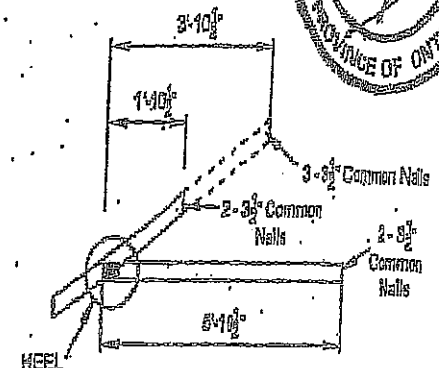
TOP CHORD LIVE LOAD	:	39.4 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	10.5 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F
TOTAL LOAD		59.9 P.S.F.

DWG NO TIN 2495.14
STRUCTURAL COMPONENT ONLY

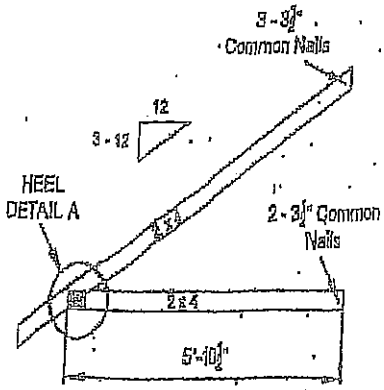
LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO



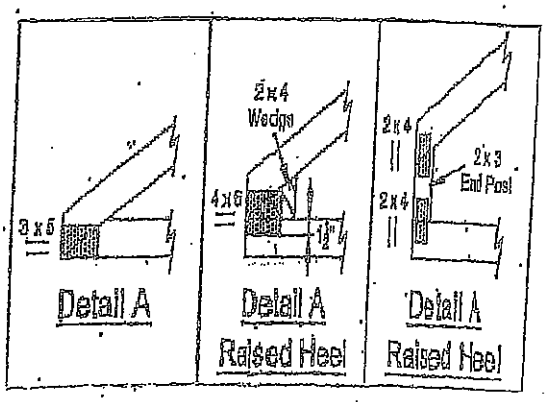
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)

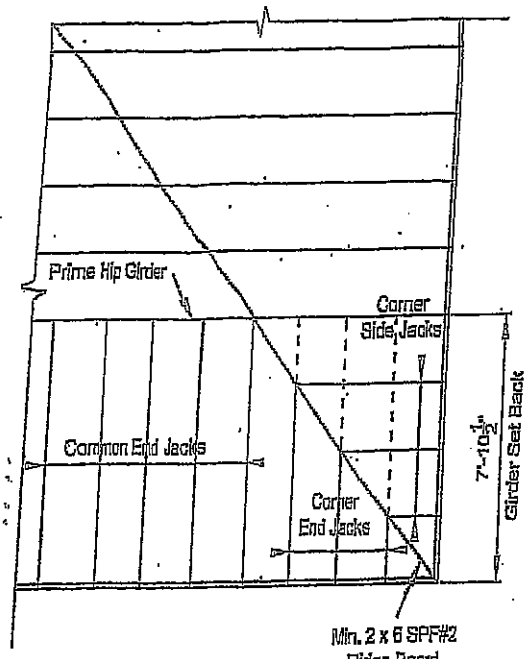
MICRO CITY

ENGINEERING SERVICES INC. TEL: (519) 287.- 2242

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

LUMBER SPECIFICATION

TOP CHORD : 2x4 SPF#2
BOTTOM CHORD : 2x4 SPF#2
WEBS : 2x3 SPF#2
UNLESS OTHERWISE SHOWN



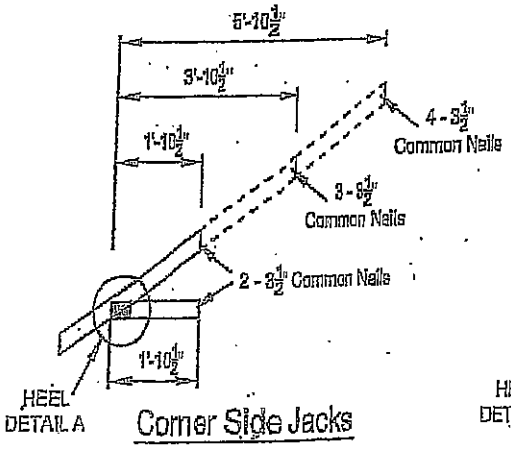
45° Hip End

DESIGN LOAD:

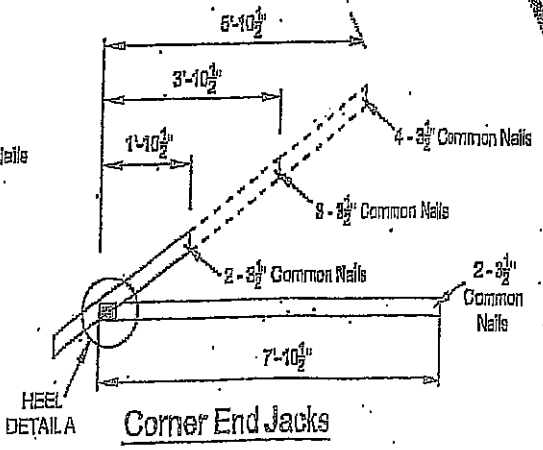
TOP CHORD LIVE LOAD	: 34.8 P.S.F.
TOP CHORD DEAD LOAD	: 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	: 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	: 7.0 P.S.F.
TOTAL LOAD	: 44.8 P.S.F.

DWG NO TRM 3503 14
STRUCTURAL
COMPONENT ONLY

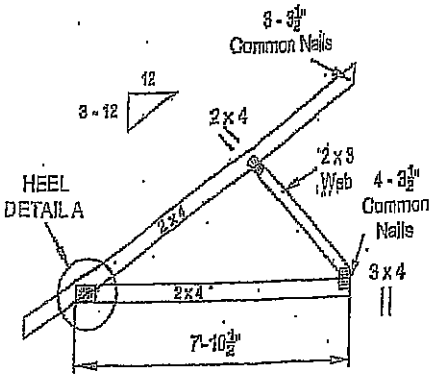
LICENSED PROFESSIONAL ENGINEER
1-2014
S. KATSOULAKOS
PROVINCE OF ONTARIO



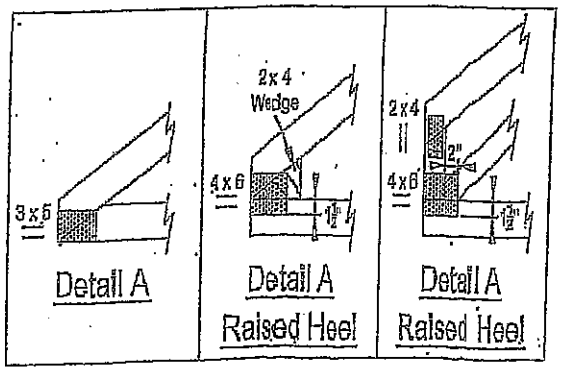
Corner Side Jacks



Corner End Jacks



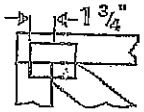
Common End Jacks



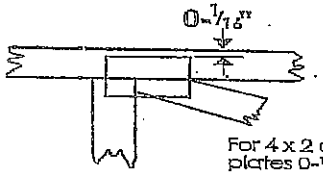
NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

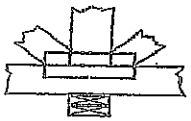
4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

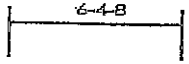
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses

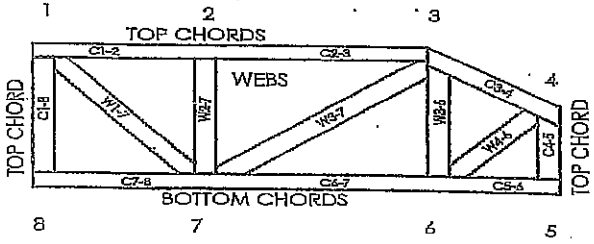
DSB-89: *Design Standard for Bracing.*

BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

MiTek®

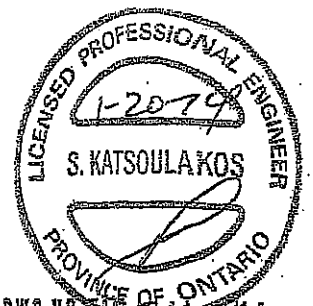
POWER TO PERFORM.™

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

Micro City Engineering Services Inc.
(BCIN: 26064; FIRM BCIN: 29991)
RR #1, Po Box 61
Glencoe, Ontario
N0L 1M0
(519) 287 - 2242; Fax: (519) 287 - 5750 (Call)



Responsibilities:

Micro City Engineering Services is responsible for the design of trusses as individual components.

It is the responsibilities of others to ascertain that the design loads utilized on this (these) drawing(s) meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdiction over such decisions.

All dimensions are to be verified by the owner, contractor, architect, or other authority having input over such decisions prior to truss component manufacture. At no time shall Micro City Engineering Services Inc. or its employees be responsible for dimension errors.

Micro City Engineering Services Inc. bears no responsibility for the erection of any truss components. Persons erecting truss components are cautioned to seek professional advice regarding temporary and permanent bracing systems and to be totally familiar with all aspects of truss erection prior to proceeding on any truss component erection job. Any bracing shown on Micro City Engineering Services Inc. or Tamarack Roof Trusses Inc. sealed or unsealed truss component drawings is specified for the single truss component in question and is identified as an integral part of the design for that particular truss component but is not meant to represent the only required bracing for that particular truss component when installed as a component in a series of truss components in a roof truss system.

~~It is the truss manufacturer's responsibility to ensure that trusses are manufactured in accordance with Micro City Engineering~~ Services Inc. specifications outlined below:

SPECIFICATIONS:

Truss components sealed by Micro City Engineering Services Inc. must conform to the relevant sections of the current Building Code of Ontario and Canada (Part 4 or Part 9) or the current Farm Building Code of Canada in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the Truss Plate Institute of Canada (TPIC). All unit lumber and nailing stresses identified on truss component design drawings and/or used in the design of individual truss components shall conform to the current CSA Wood Design standard identified in the current Building Code and TPIC Design Standards.

The lumber used to manufacture any truss component is to conform to the specified size and grade identified on the truss drawing.

The lumber used in the manufacture of any truss component is not to exceed 19% during its service use unless specifically noted on the truss drawing.

The lumber used in the manufacture of any truss component is not to be treated with any chemicals during its service life unless specifically noted on the truss drawing.

Connector plates shall be applied to both faces of the truss component at each joint and shall be positioned exactly as specified.

The top chord of any truss component is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the sealed truss component drawing but not exceeding 24" o/c (Part 9 design) and not exceeding 48" o/c (Part 4 or Agricultural design).

When a truss component is to be installed with no rigid ceiling attached directly to the bottom chord, then the bottom chord is to be laterally braced at intervals not exceeding 3m (or 10'-0").

All sealed or unsealed truss component drawings provided by Micro City Engineering Services Inc. Or Tamarack Roof Trusses Inc. should be read in conjunction with the following:

Warning-Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473C rev 10-'08 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer - not the truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component Safety Information available from the Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA, 22314.