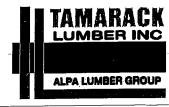


	Products				
PlotID	Length	Product	Plies	Net Qty	
J1	18-00-00	11 7/8" NI-40x	1	10	
J2	16-00-00	11 7/8" NI-40x	1	20	
J3	14-00-00	11 7/8" NI-40x	1	12	
J3DJ	14-00-00	11 7/8" NI-40x	2	4	
J4	12-00-00	11 7/8" NI-40x	1	2	
J5	4-00-00	11 7/8" NI-40x	1	1	
J6	2-00-00	11 7/8" NI-40x	1	4	
B3	12-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	3	3	
B1	8-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	3	3	
B9	6-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1	
B4	6-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2 .	2	
B2	2-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1	

	Connector Summary				
Qty	Manuf	Product			
4	H1	IUS2.56/11.88			
2	H1	IUS2.56/11.88			
12	H1	IUS2.56/11.88			
6	H1	IUS2.56/11.88			



BUILDER: GREENPARK HOMES

SITE: RUSSELL GARDENS

MODEL: ROSEWOOD 1A

ELEVATION: 1

LOT:

CITY: WATERDOWN

SALESMAN: M D DESIGNER: AJ REVISION:

NOTES:

REFER TO THE NORDIC INSTALLATION GUIDE FOR PROPER STORAGE AND INSTALLATION. **SQUASH BLOCKS OF 2x4, 2x6, 2x8 #2** S.P.F REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. CANTILEVERED JOISTS INCLUDING CANT' OVER BRICK REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR HOLES INCLUDING DUCT CHASE AND FIELD CUT OPENINGS SEE FIGURE 7, TABLES 1 & 2. CERAMIC TILE APPLICATION AS PER O.B.C 9.30.6. LOADING:

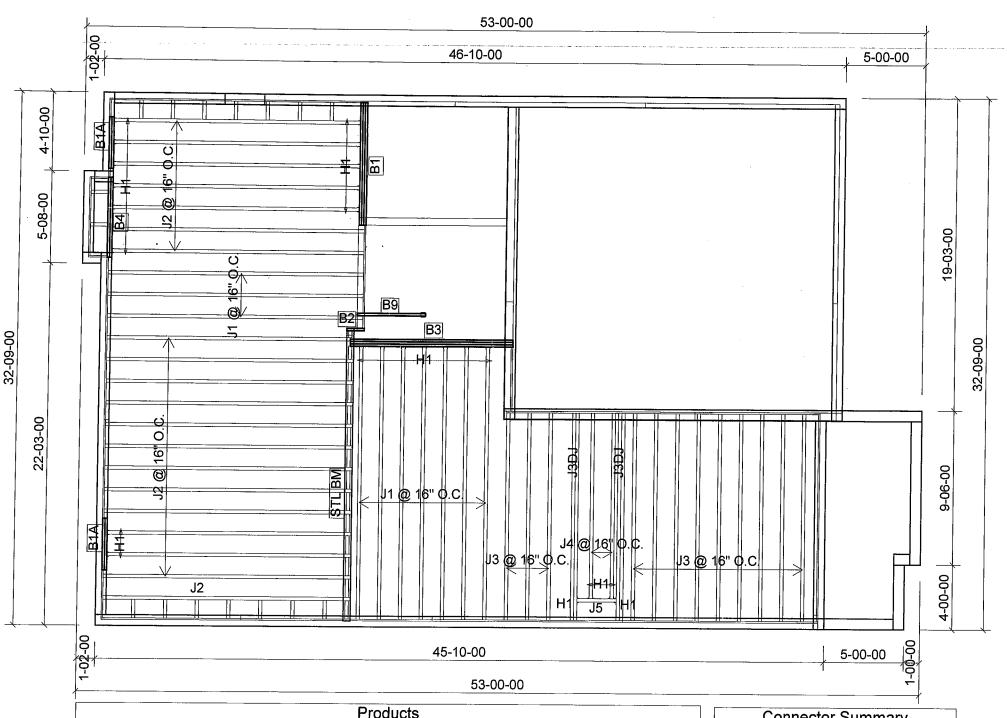
DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft² DEAD LOAD: 15.0 lb/ft TILED AREAS: 20 lb/ft

SUBFLOOR: 3/4" GLUED AND NAILED

DATE: 8/21/2017

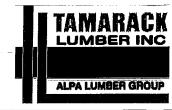
1st FLOOR

STANDARD



		Products		
PlotID	Length	Product	Plies	Net Qty
J1	18-00-00	11 7/8" NI-40x	1	10
J2	16-00-00	11 7/8" NI-40x	1	20
J3	14-00-00	11 7/8" NI-40x	. 1	12
J3DJ	14-00-00	11 7/8" NI-40x	2	4
J4	12-00-00	11 7/8" NI-40x	1	2
J5	4-00-00	11 7/8" NI-40x	1	1
B3	12-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	3	3
B1	8-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	3	3
B9	6-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
B4	6-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2
B1A	4-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	4
B2	2-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1

38
88
8
88



BUILDER: GREENPARK HOMES

SITE: RUSSELL GARDENS

MODEL: ROSEWOOD 1A

ELEVATION: 1

LOT:

CITY: WATERDOWN

SALESMAN: M D DESIGNER: AJ REVISION:

NOTES:

REFER TO THE NORDIC **INSTALLATION** GUIDE FOR PROPER STORAGE AND INSTALLATION. SQUASH BLOCKS OF 2x4, 2x6, 2x8 #2 S.P.F REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. CANTILEVERED JOISTS INCLUDING CANT' OVER BRICK REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR HOLES INCLUDING DUCT CHASE AND FIELD CUT OPENINGS SEE FIGURE 7, TABLES 1 & 2. **CERAMIC TILE APPLICATION AS PER** O.B.C 9.30.6.

DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft² DEAD LOAD: 15.0 lb/ft TILED AREAS: 20 lb/ft

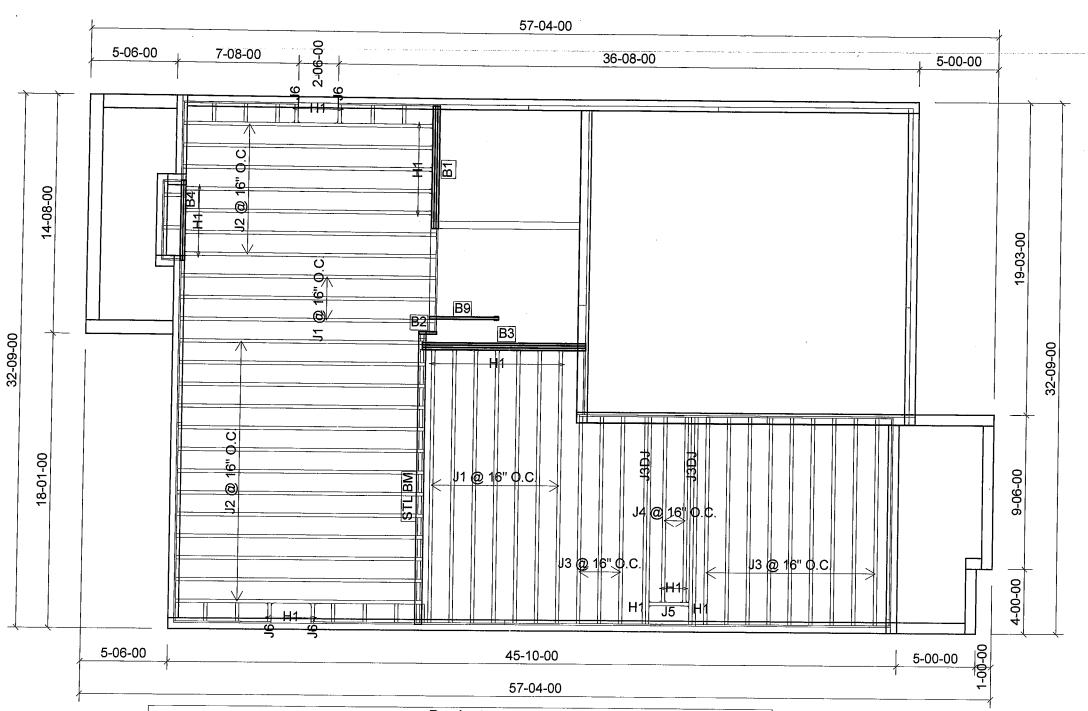
SUBFLOOR: 3/4" GLUED AND NAILED

DATE: 8/21/2017

LOADING:

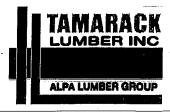
1st FLOOR

DECK CONDITION



-,,		Products		
PlotID	Length	Product	Plies	Net Qty
J1	18-00-00	11 7/8" NI-40x	1	10
J2	16-00-00	11 7/8" NI-40x	1	20
J3	14-00-00	11 7/8" NI-40x	1	12
J3DJ	14-00-00	11 7/8" NI-40x	2	4
J4	12-00-00	11 7/8" NI-40x	1	2
J5	4-00-00	11 7/8" NI-40x	1	1
J6	2-00-00	11 7/8" NI-40x	1	4
B3	12-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	3	3
B1	8-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	3	3
B9	6-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
B4	6-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2
B2	2-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1

	Connector Summary				
Qty	Manuf	Product			
4	H1	IUS2.56/11.88			
2	H1	IUS2.56/11.88			
12	H1	IUS2.56/11.88			
6	H1	IUS2.56/11.88			



BUILDER: GREENPARK HOMES

SITE: RUSSELL GARDENS

MODEL: ROSEWOOD 1A

ELEVATION: 1

LOT:

CITY: WATERDOWN

SALESMAN: M D DESIGNER: AJ REVISION:

NOTES:

REFER TO THE NORDIC **INSTALLATION GUIDE FOR PROPER** STORAGE AND INSTALLATION. **SQUASH BLOCKS OF 2x4, 2x6, 2x8 #2** S.P.F REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. CANTILEVERED JOISTS INCLUDING CANT' OVER BRICK REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR HOLES INCLUDING DUCT CHASE AND FIELD CUT OPENINGS SEE FIGURE 7, TABLES 1 & 2. **CERAMIC TILE APPLICATION AS PER** O.B.C 9.30.6.

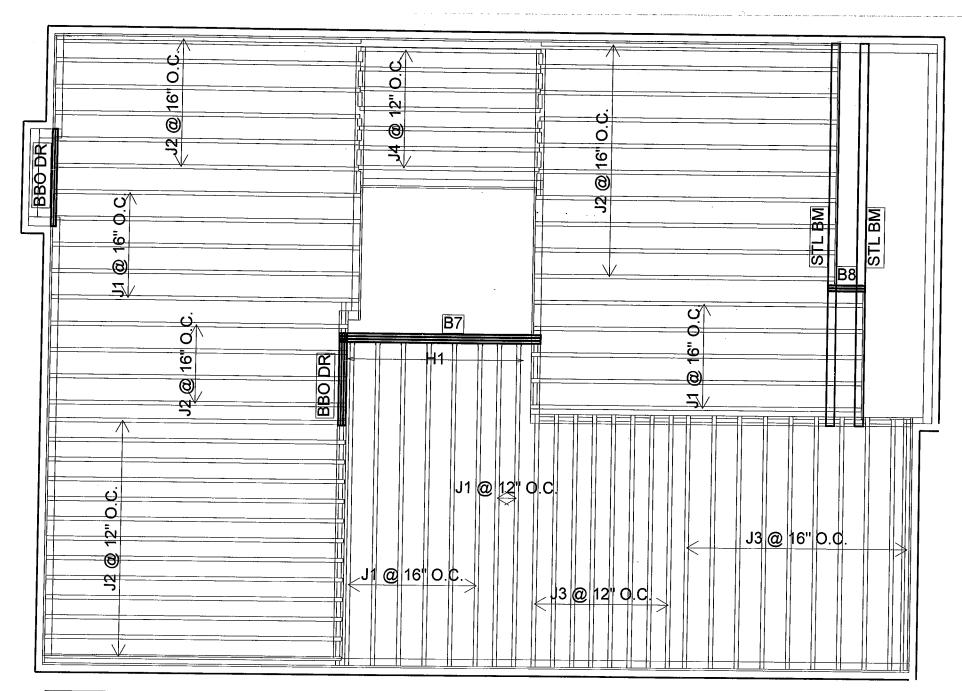
LOADING: DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft² DEAD LOAD: 15.0 lb/ft TILED AREAS: 20 lb/ft

SUBFLOOR: 3/4" GLUED AND NAILED

DATE: 8/21/2017

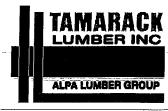
1st FLOOR

WALK UP CONDITION



	Products					
PlotID	Length	Product	Plies	Net Qty		
J1	18-00-00	11 7/8" NI-40x	1	18		
J2	16-00-00	11 7/8" NI-40x	1	33		
J3	14-00-00	11 7/8" NI-40x	1	18		
J4	10-00-00	11 7/8" NI-40x	1	7		
B7	12-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	3	3		
B8	2-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2		

	Connector Summary				
Qty Manuf Product					
1	H1	IUS2.56/9.5			
7	H1	IUS2.56/11.88			



BUILDER: GREENPARK HOMES

SITE: RUSSELL GARDENS

MODEL: ROSEWOOD 1A

ELEVATION: 1

LOT:

CITY: WATERDOWN

SALESMAN: M D DESIGNER: AJ REVISION:

NOTES:

REFER TO THE NORDIC **INSTALLATION GUIDE FOR PROPER** STORAGE AND INSTALLATION. **SQUASH BLOCKS OF 2x4, 2x6, 2x8 #2** S.P.F. REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. CANTILEVERED JOISTS INCLUDING CANT' OVER BRICK REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURE 7 TABLES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR HOLES INCLUDING DUCT CHASE AND FIELD CUT OPENINGS SEE FIGURE 7 TABLES 1 & 2 OF THE INSTALLATION GUIDE. CERAMIC TILE APPLICATION AS PER O.B.C. 9.30.6 LOADING:

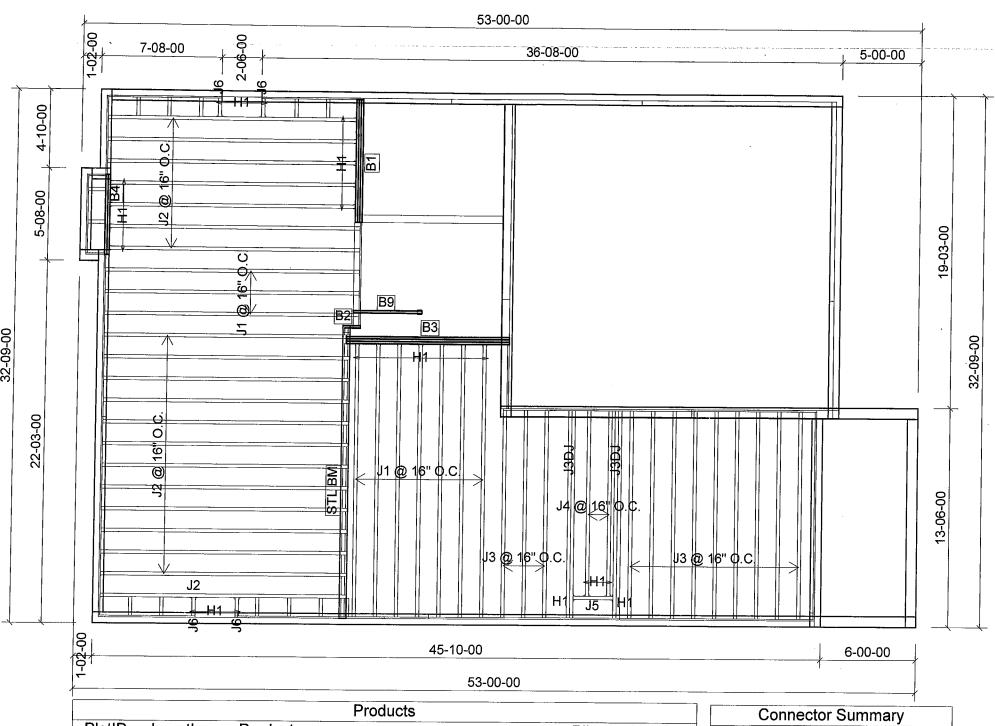
DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft² DEAD LOAD: 15.0 lb/ft TILED AREAS: 20 lb/ft

SUBFLOOR: 5/8" GLUED AND NAILED

DATE: 8/21/2017

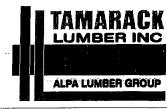
2nd FLOOR

STANDARD



		Products		
PlotID	Length	Product	Plies	Net Qty
J1	18-00-00	11 7/8" NI-40x	1	10
J2	16-00-00	11 7/8" NI-40x	1	20
J3	14-00-00	11 7/8" NI-40x	1	12
J3DJ	14-00-00	11 7/8" NI-40x	2	4
J4	12-00-00	11 7/8" NI-40x	1	2
J5	4-00-00	11 7/8" NI-40x	1	1
J6	2-00-00	11 7/8" NI-40x	1	4
B3	12-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	3	3
B1	8-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	3	3
B9	6-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
B4	6-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2
B2	2-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1

	Connector Summary			
Qty	Manuf	Product		
4	H1	IUS2.56/11.88		
2	H1	IUS2.56/11.88		
12	H1	IUS2.56/11.88		
6	H1	IUS2.56/11.88		



BUILDER: GREENPARK HOMES

SITE: RUSSELL GARDENS

MODEL: ROSEWOOD 1A

ELEVATION: 2,3

LOT:

CITY: WATERDOWN

SALESMAN: M D DESIGNER: AJ REVISION:

NOTES:

REFER TO THE NORDIC **INSTALLATION GUIDE FOR PROPER** STORAGE AND INSTALLATION. SQUASH BLOCKS OF 2x4, 2x6, 2x8 #2 S.P.F REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS, SEE FIGURE 1. CANTILEVERED JOISTS INCLUDING CANT' OVER BRICK REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR HOLES INCLUDING DUCT CHASE AND FIELD CUT OPENINGS SEE FIGURE 7, TABLES 1 & 2. **CERAMIC TILE APPLICATION AS PER** O.B.C 9.30.6.

LOADING:
DESIGN LOADS: L/480.000
LIVE LOAD: 40.0 lb/ft²
DEAD LOAD: 15.0 lb/ft
TILED AREAS: 20 lb/ft

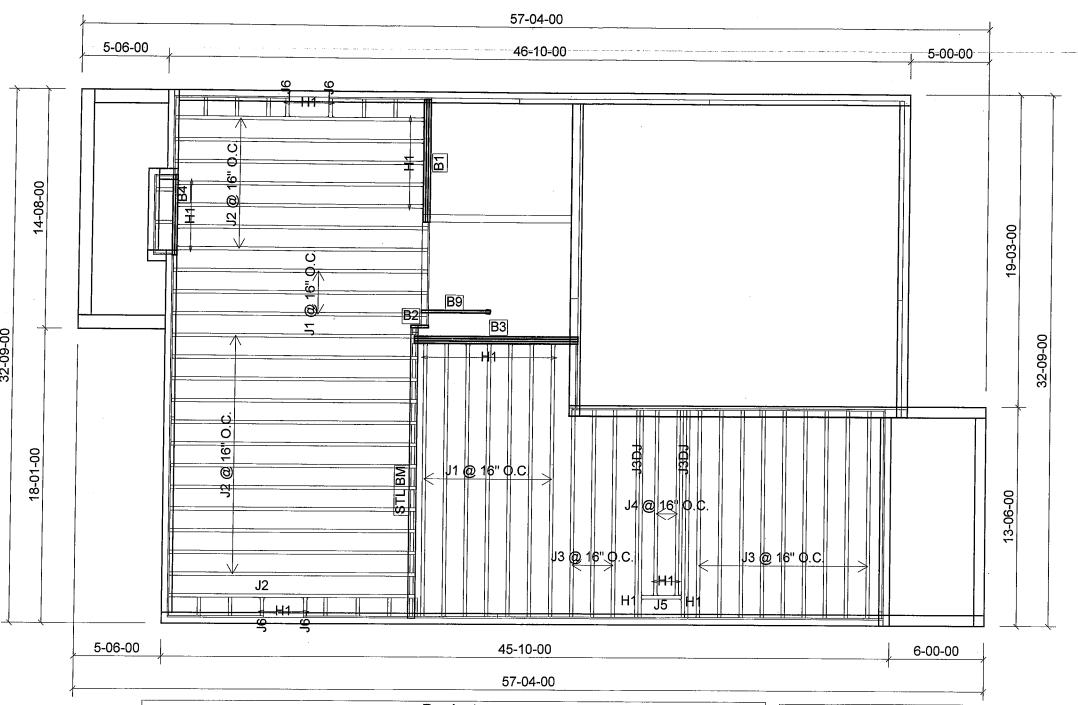
TILED AREAS: 20 ID/IT

SUBFLOOR: 3/4" GLUED AND NAILED

DATE: 8/21/2017

1st FLOOR

STANDARD



	Products				
PlotID	Length	Product	Plies	Net Qty	
J1	18-00-00	11 7/8" NI-40x	1	10	
J2	16 - 00-00	11 7/8" NI-40x	1	20	
J3	14-00-00	11 7/8" NI-40x	1	12	
J3DJ	14-00-00	11 7/8" NI-40x	2	4	
J4	12-00-00	11 7/8" NI-40x	1	2	
J5	4-00-00	11 7/8" NI-40x	1	1	
J6	2-00-00	11 7/8" NI-40x	1	4	
B3	12-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	3	3	
B1	8-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	3	3	
B9	6-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1	
B4	6-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2	
B2	2-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1	

	Connector Summary			
Qty	Manuf	Product		
4	H1	IUS2.56/11.88		
2	H1	IUS2.56/11.88		
12	H1	IUS2.56/11.88		
6	H1	IUS2.56/11.88		



BUILDER: GREENPARK HOMES

SITE: RUSSELL GARDENS

MODEL: ROSEWOOD 1A

ELEVATION: 2,3

LOT:

CITY: WATERDOWN

SALESMAN: M D DESIGNER: AJ REVISION:

NOTES:

REFER TO THE NORDIC **INSTALLATION GUIDE FOR PROPER** STORAGE AND INSTALLATION. SQUASH BLOCKS OF 2x4, 2x6, 2x8 #2 S.P.F REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. CANTILEVERED JOISTS INCLUDING CANT' OVER BRICK REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR HOLES INCLUDING DUCT CHASE AND FIELD CUT OPENINGS SEE FIGURE 7, TABLES 1 & 2. **CERAMIC TILE APPLICATION AS PER** O.B.C 9.30.6.

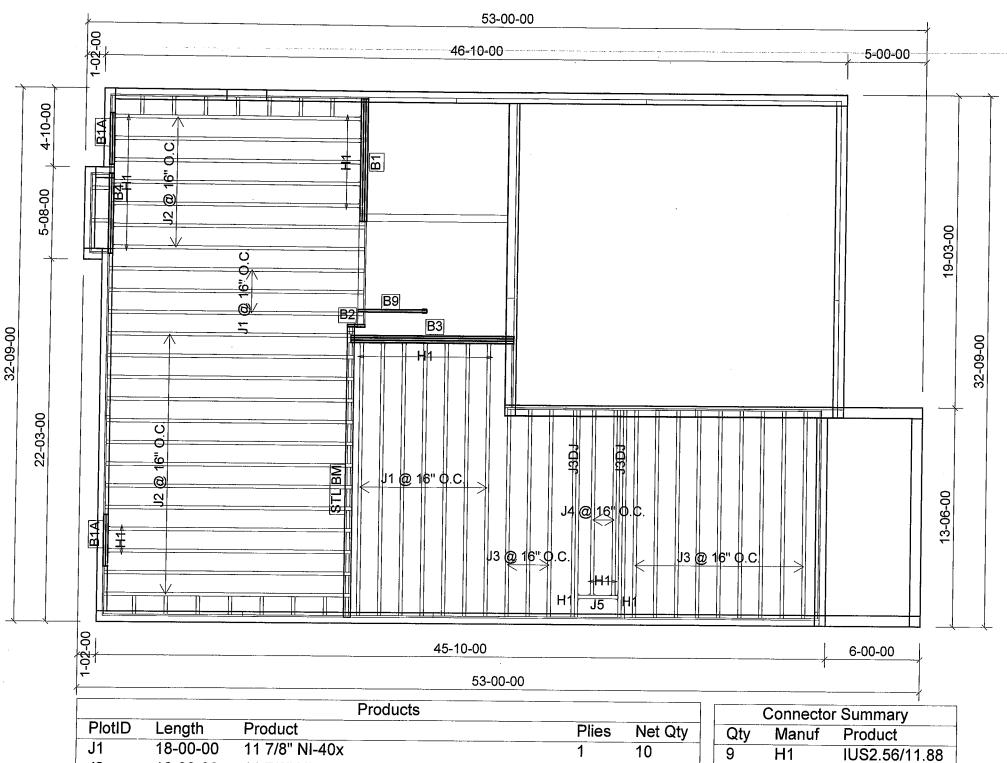
LOADING:
DESIGN LOADS: L/480.000
LIVE LOAD: 40.0 lb/ft²
DEAD LOAD: 15.0 fb/ft
TILED AREAS: 20 lb/ft

SUBFLOOR: 3/4" GLUED AND NAILED

DATE: 8/21/2017

1st FLOOR

WALK UP CONDITON



Products							
PlotID	Length	Product	Plies	Net Qty			
J1	18-00-00	11 7/8" NI-40x	1	10			
J2	16-00-00	11 7/8" NI-40x	1	20			
J3	14-00-00	11 7/8" NI-40x	1	12			
J3DJ	14-00-00	11 7/8" NI-40x	2	4			
J4	12-00-00	11 7/8" NI-40x	1	2			
J5	4-00-00	11 7/8" NI-40x	1	1			
B3	12-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	3	3			
B1	8-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	3	3			
B9	6-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1			
B4	6-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2			
B1A	4-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	4			
B2	2-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1			

Connector Summary						
Qty	Manuf	Product				
9	H1	IUS2.56/11.88				
2	H1	IUS2.56/11.88				
12	H1	IUS2.56/11.88				
2	H1	IUS2.56/11.88				



BUILDER: GREENPARK HOMES

SITE: RUSSELL GARDENS

MODEL: ROSEWOOD 1A

ELEVATION: 2,3

LOT:

CITY: WATERDOWN

SALESMAN: M D DESIGNER: AJ REVISION:

NOTES:

REFER TO THE NORDIC **INSTALLATION GUIDE FOR PROPER** STORAGE AND INSTALLATION. SQUASH BLOCKS OF 2x4, 2x6, 2x8 #2 S.P.F REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. CANTILEVERED JOISTS INCLUDING CANT' OVER BRICK REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR HOLES INCLUDING DUCT CHASE AND FIELD CUT OPENINGS SEE FIGURE 7, TABLES 1 & 2. **CERAMIC TILE APPLICATION AS PER** O.B.C 9.30.6.

LOADING:
DESIGN LOADS: L/480.000

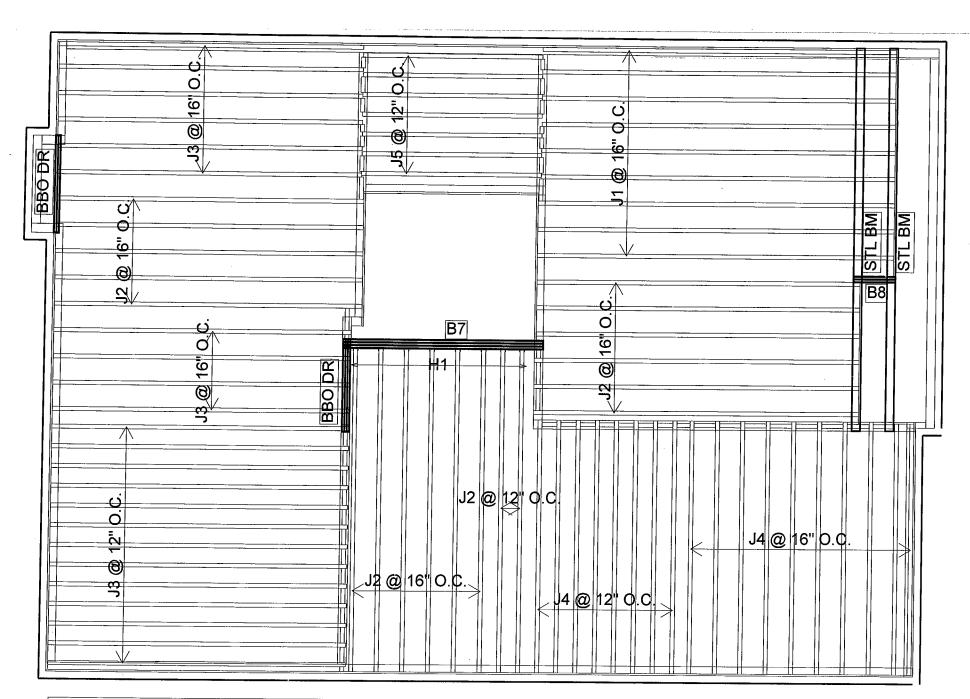
LIVE LOAD: 40.0 lb/ft² DEAD LOAD: 15.0 lb/ft TILED AREAS: 20 lb/ft

SUBFLOOR: 3/4" GLUED AND NAILED

DATE: 8/21/2017

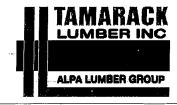
1st FLOOR

DECK CONDITON



Products							
PlotID	Length	Product	Plies	Net Qty			
J1	20-00-00	11 7/8" NI-40x	1	9			
J2	18-00-00	11 7/8" NI-40x	1	19			
J3	16-00-00	11 7/8" NI-40x	1	23			
J4	14-00-00	11 7/8" NI-40x	1	18			
J5	10-00-00	11 7/8" NI-40x	1	7			
B7	12-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	3	3			
B8	4-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2			

	Connector Summary							
Qty	Manuf	Product						
1	H1	IUS2.56/9.5						
7	H1	IUS2.56/11.88						



BUILDER: GREENPARK HOMES

SITE: RUSSELL GARDENS

MODEL: ROSEWOOD 1A

ELEVATION: 2

LOT:

CITY: WATERDOWN

SALESMAN: M D DESIGNER: AJ REVISION:

NOTES:

REFER TO THE NORDIC **INSTALLATION GUIDE FOR PROPER** STORAGE AND INSTALLATION. **SQUASH BLOCKS OF 2x4, 2x6, 2x8 #2** S.P.F. REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. CANTILEVERED JOISTS INCLUDING CANT' OVER BRICK REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURE 7 TABLES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR HOLES INCLUDING DUCT CHASE AND FIELD CUT OPENINGS SEE FIGURE 7 TABLES 1 & 2 OF THE INSTALLATION GUIDE. CERAMIC TILE APPLICATION AS PER O.B.C. 9.30.6 LOADING:

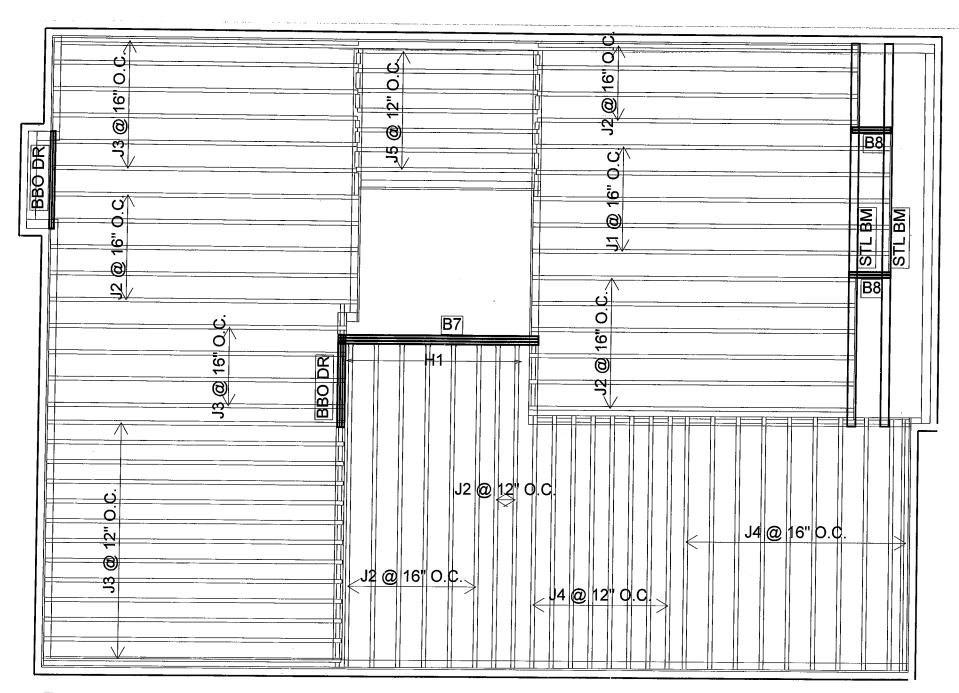
DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft² DEAD LOAD: 15.0 lb/ft TILED AREAS: 20 lb/ft

SUBFLOOR: 5/8" GLUED AND NAILED

DATE: 8/21/2017

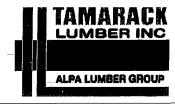
2nd FLOOR

STANDARD



Products							
PlotID	Length	Product	Plies	Net Qty			
J1	20-00-00	11 7/8" NI-40x	1	5			
J2	18-00-00	11 7/8" NI-40x	1	23			
J3	16-00-00	11 7/8" NI-40x	1	23			
J4	14-00-00	11 7/8" NI-40x	1	18			
J5	10-00-00	11 7/8" NI-40x	1	7			
B7	12-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	3	3			
B8	4-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	4			

	Connector Summary							
Qty	Manuf	Product						
1	H1	IUS2.56/9.5						
7	H1	IUS2.56/11.88						



BUILDER: GREENPARK HOMES

SITE: RUSSELL GARDENS

MODEL: ROSEWOOD 1A

ELEVATION: 3

LOT:

CITY: WATERDOWN

SALESMAN: M D DESIGNER: AJ REVISION:

NOTES:

REFER TO THE NORDIC **INSTALLATION GUIDE FOR PROPER** STORAGE AND INSTALLATION. **SQUASH BLOCKS OF 2x4, 2x6, 2x8 #2** S.P.F. REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. CANTILEVERED JOISTS INCLUDING CANT' OVER BRICK REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURE 7 TABLES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR HOLES INCLUDING DUCT CHASE AND FIELD CUT OPENINGS SEE FIGURE 7 TABLES 1 & 2 OF THE INSTALLATION GUIDE. CERAMIC TILE APPLICATION AS PER O.B.C. 9.30.6 LOADING:

DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft² DEAD LOAD: 15.0 lb/ft TILED AREAS: 20 lb/ft

SUBFLOOR: 5/8" GLUED AND NAILED

DATE: 8/21/2017

2nd FLOOR

STANDARD



Triple 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B1(i2554)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 21, 2017 08:11:29

Build 5033

Job Name:

Address:

City, Province, Postal Code:WATERDOWN,

Customer:

Code reports:

CCMC 12472-R

File Name: ROSEWOOD 1A EL-1.mmdl

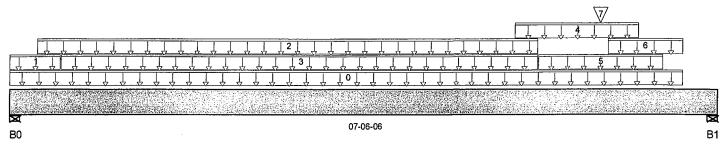
Description: Designs\Flush Beams\Basment\Flush Beams\B1(i2554)

Specifier:

Designer: Company.

Vice:

Misc:



Total Horizontal Product Length = 07-06-06

Reaction Summary (Down / Uplift) (lbs)								
Be aring	Live	De ad	Snow	Wind				
B0, 5-1/2"	3,094 / 0	1,932 / 0						
B1, 2-3/8"	2,668 / 0	1,664 / 0						

l o	ad Summary					Live	Dead	Snow	Wind	Trib.
Tag Description		Load Type Ref. Start End		1.00	0.65	1.00 1.15				
0	5(i350)	Unf. Lin. (lb/ft)	L	00-00-00	07-02-00		81		.,	n/a
1	5(i350)	Unf. Lin. (lb/ft)	L	00-00-00	00-06-08	311	155			n/a
2	Smoothed Load	Unf. Lin. (lb/ft)	L	00-03-08	05-07-08	328	164			n/a
3	5(i350)	Unf. Lin. (lb/ft)	L	00-06-08	05-07-08	517	258			n/a
4	5(i350)	Unf. Lin. (lb/ft)	L	05-04-08	06-08-08	141	70			n/a
5	5(i350)	Unf. Lin. (lb/ft)	L	05-07-08	06-11-08	292	146			n/a
6	5(i350)	Unf. Lin. (lb/ft)	L	06-04-08	07-02-00	187	94			n/a
7	J2(i2558)	Conc. Pt. (lbs)	L	06-03-08	06-03-08	475	237			n/a

	Factored	Factored	Demand /	Load	Location
Controls Summary	Dem and	Resistance	Resistance	Case	
Pos. Moment	11,827 ft-lbs	60,415 ft-lbs	19.6%	1	03-07-08
End Shear	6,101 lbs	21,696 lbs	28.1%	1	01-05-06
Total Load Defl.	L/999 (0.051")	n/a	n/a	4	03-10-08
Live Load Defl.	L/999 (0.031")	n/a	n/a	5	03-10-08
Max Defl.	0.051"	n/a	n/a	4	03-10-08
Span / Depth	7.1	n/a	n/a		00-00-00

				Demand/ Resistance	Demand/ Resistance	
Beari	ng Supports	Dim.(L x W)	Demand	Support	Member	Material
B0	Wall/Plate	5-1/2" x 5-1/4"	7,056 lbs	45.8%	20%	Unspecified
B1	Wall/Plate	2-3/8" x 5-1/4"	6,082 lbs	91.3%	40%	Unspecified

Notes

S. KATSOULAKOS E

DWO NO.TAM 42847-17
STRUCTURAL
COMPONENT ONLY



Triple 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B1(i2554)

BC CALC® Design Report



Dry 1 span | No cantilevers | 0/12 slope (deg)

August 21, 2017 08:11:29

Build 5033 Job Name:

Address:

дп кер

File Name: ROSEWOOD 1A EL-1.mmdl

Description: Designs\Flush Beams\Basment\Flush Beams\B1(i255

Specifier:

Designer: Company:

Misc:

City, Province, Postal Code:WATERDOWN, Customer:

Code reports:

CCMC 12472-R

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

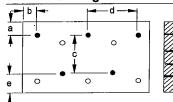
BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA

CONFORMS TO OBC 2012

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Connection Diagram



4 rous

a minimum = **3**" c = 6²-7/8" b minimum = 3" d = **3**" d + *1*" e minimum = **3**".

Calculated Side Load = 629.0 lb/ft

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record. Nailing schedule applies to both sides of the member.

Connectors are: 16d A Nails

3½" ARDOX SPIRAL

Disclosure

Completeness and accuracy of input must be verified by anyone w ho w ould rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered w ood products must be in accordance w ith current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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DWO NO.TAM 42849-17 STRUCTURAL COMPONENT ONLY



Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

PASSED

Basment\Flush Beams\B1A(i2733)

BC CALC® Design Report

Dry | 1 span | No cant.

March 14, 2018 14:51:19

Build 6215

Job name: Address:

Customer: Code reports:

City, Province, Postal Code: WAT...WN

CCMC 12472-R

ROSEWOOD 1A EL-1 WOD.mmdl

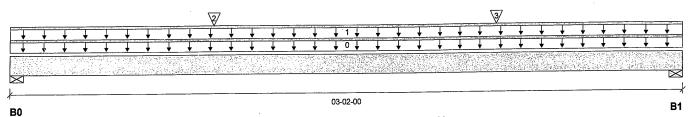
File name: Basment\Flush Beams\B1A(i2733) Description:

Wind

Specifier:

ΑJ Designer:

Company:



Total Horizontal Product Length = 03-02-00

Snow

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead				
B0, 3-1/2"	904 / 0	600 / 0				
B1. 3-1/2"	930 / 0	613 / 0				

10	ad Summary					Live	Dead	Snow	Wind	Tributary
	Description	Load Type	Ref.	Start	End	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	03-02-00		12			00-00-00
1	E1(i207)	Unf. Lin. (lb/ft)	L	00-00-00	03-02-00	315	239			n\a
2	J2(i2723)	Conc. Pt. (lbs)	L	00-11-08	00-11-08	418	209			n\a
3	J2(i2728)	Conc. Pt. (lbs)	L	02-03-08	02-03-08	418	209			n\a

		Factored	Demand/	_	
Controls Summary	Factored Demand	Resistance	Resistance	Case	Location
Pos. Moment	1,331 ft-lbs	35,392 ft-lbs	3.8%	1	01-06-09
End Shear	808 lbs	14,464 l bs	5.6%	1	01-03-06
Total Load Deflection	L/999 (0.001")	n\a	n\a	4	01-07-00
Live Load Deflection	L/999 (0.001")	n\a	n\a	5	01-07-00
Max Defl.	0.001"	n\a	n\a	4	01-07-00
Span / Depth	2.7				

Beari	ing Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	3-1/2" x 3-1/2"	2,105 lbs	40.2%	14.1%	Unspecified
B1	Wall/Plate	3-1/2" x 3-1/2"	2,160 lbs	41.3%	14.5%	Unspecified

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NB

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record.







Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

PASSED

Basment\Flush Beams\B1A(i2733)

BC CALC® Design Report

Dry | 1 span | No cant.

March 14, 2018 14:51:19

Build 6215

Job name: Address:

\\/\T \\/\N

File name: Description:

ROSEWOOD 1A EL-1 WOD.mmdl Basment\Flush Beams\B1A(i2733)

City, Province, Postal Code: WAT...WN Customer:

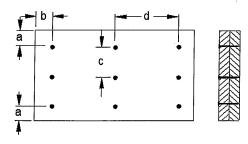
VN Specifier: Designer:

Specifier:
Designer: AJ

Code reports:

CCMC 12472-R Company:

Connection Diagram



a minimum = 2" b minimum = 3" c=4" d= 2 4"

Calculated Side Load = 561.0 lb/ft

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record.

Connectors are: 16de and Nails

A 3-1/2" ARDOX SPIRAL

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DWG NO. TAM 15035.48
STRUCTURAL
COMPONENT ONLY

Disclosure

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

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Single 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B2(i2556)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 21, 2017 08:10:56

Build 5033

Job Name:

Address:

City, Province, Postal Code:WATERDOWN,

Customer:

Code reports:

CCMC 12472-R

File Name: ROSEWOOD 1A EL-1.mmdl

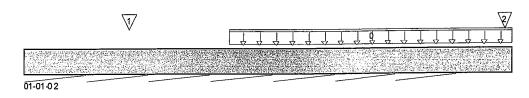
Description: Designs\Flush Beams\Basment\Flush Beams\B2(i2556)

Specifier:

Designer:

Company:

Misc:



Total Horizontal Product Length = 01-01-02

Reaction Summary (Down Bearing	/ Uplift) (lbs) Live	De ad	S	now	Win	d			
Load Summary Tag Description	Load Type	Re f.	. Start	En d	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Trib.
0 6(i351) 1 - 2 6(i351)	Unf. Lin. (lb/ft) Conc. Pt. (lbs) Conc. Pt. (lbs)	L	00-05-08 00-02-13 01-00-14	01-01-02 00-02-13 01-00-14	55	81 39 4			n/a n/a n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Distributed Load(B0)	113 lb/ft	18,735 lb/ft	0.6%	0	n/a
Concentrated Load (B0)	123 lbs	8,407 lbs	1.5%	0	n/a

CONFORMS TO OBC 2012

Disclosure

Completeness and accuracy of input must be verified by anyone w ho w ould rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered w ood products must be in accordance w ith current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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DWG NO.TAM 4265017 STRUCTURAL COMPONENT ONLY



Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP Basment\...\B4(i2557)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 21, 2017 08:10:57

BC CALC® Design Report



Build 5033

Job Name: Address:

City, Province, Postal Code:WATERDOWN,

Customer:

Code reports:

CCMC 12472-R

File Name: ROSEWOOD 1A EL-1.mmdl

Description: Designs\Flush Beams\Basment\Flush Beams\B4(i2557)

Specifier:

Designer:

Company: Misc:

2/		3
⊠ B0	04-11-00	B1

Total Horizontal Product Length = 04-11-00

Reaction Summary (Down / Uplift) (Ibs)									
Bearing	Live	Dead	Snow	Wind					
B0, 3-1/2"	1,791 / 0	977/0							
B1, 3-1/2"	1,682 / 0	922/0							

Lo	oad Summary					Live	Dead	Snow	Wind	Trib.
Tag Description		Load Type	Re	f. Start	En d	1.00	0.65	1.00	1.15	
0	Smoothed Load	Unf. Lin. (lb/ft)	L	00-00-00	04-11-00	346	172			n/a
1	FC1 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	04-00-00	26	13			n/a
2	E1 (i207)	Conc. Pt. (lbs)	L	00-02-12	00-02-12	865	484			n/a
3	E4(i204)	Conc. Pt. (lbs)	L	04-08-04	04-08-04	799	451			n/a

CONFORMS TO OBC 2012

	Factored	Factored	Demand /	Load	Location
Controls Summary	Demand	Resistance	Resistance	Case	
Pos. Moment	1,795 ft-lbs	38,727 ft-lbs	4.6%	1	02-11-06
End Shear	1,175 lbs	14,464 lbs	8.1%	1	03-07-10
Total Load Defl.	L/999 (0.005")	n/a	n/a	4	02-05-11
Live Load Defl.	L/999 (0.003")	n/a	n/a	5	02-05-11
Max Defl.	0.005"	n/a	n/a	4	02-05-11
Span / Depth	4.5	n/a	n/a		00-00-00

				Resistance	Resistance	
Beari	ng Supports	Dim.(LxW)	Demand	Support	Member	Material
B0	Wall/Plate	3-1/2" x 3-1/2"	3,909 lbs	74.7%	26.2%	Unspecified
B1	Wall/Plate	3-1/2" x 3-1/2"	3,675 lbs	70.2%	24.6%	Unspecified

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA

O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

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DWO NO . TAN 4285/17 STRUCTURAL COMPONENT ONLY



Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP Basment\...\B4(i2557)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 21, 2017 08:10:57

BC CALC® Design Report

File Name: ROSEWOOD 1A EL-1.mmdl

Description: Designs\Flush Beams\Basment\Flush Beams\B4(i255)

Specifier: Designer:

Company. Misc:

Build 5033

Job Name: Address:

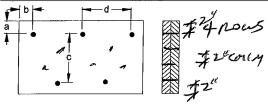
City, Province, Postal Code:WATERDOWN,

Customer:

Code reports:

CCMC 12472-R

Connection Diagram



a minimum = 2" c = 7-7/8" b minimum = 3" d = 🕬

Calculated Side Load = 730.7 lb/ft

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record.

Connectors are: 16d A Nails
3½ ARDOX SPIRAL

Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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> POLINCE OF ONTE DWG NO. TAM 4285/-17

STRUCTURAL COMPONENT ONLY



Single 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B9(i2520)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 21, 2017 08:10:57

Build 5033

Job Name: Address:

City, Province, Postal Code:WATERDOWN,

Customer:

Code reports:

CCMC 12472-R

File Name: ROSEWOOD 1A EL-1.mmdl

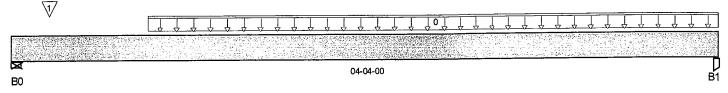
Description: Designs\Flush Beams\Basment\Flush Beams\B9(i2520)

Specifier:

Misc:

Designer:

Company:



Total Horizontal Product Length = 04-04-00

Bearing	(Down / Uplift) (lbs) Live	De ad	Snow	Wind	
B0, 5-1/2"	397/0	224/0			
B1, 3-1/2"	494/0	259/0			

١٥	ad Summary			Live	Dead	Snow Wind	i rib.
	g Description	Load Type	Ref. Start	En d 1.00	0.65	1.00 1.15	
0	Us er Load	Unf. Lin. (lb/ft)	L 00-10-00	04-04-00 240	120		n/a
1	5(i350)	Conc. Pt. (lbs)	L 00-02-12	00-02-12 49	36		n/a

	Factored	Factored	Demand /	Load	Location
Controls Summary	Dem and	Resistance	Resistance	Case	
Pos. Moment	865 ft-lbs	13,146 ft-lbs	6.6%	1	02-03-10
End Shear	742 lbs	7,232 lbs	10.3%	1	01-05-06
Total Load Defl.	L/999 (0.003")	n/a	n/a	4	02-03-00
Live Load Defl.	L/999 (0.002")	n/a	n/a	5	02-03-00
Max Defl.	0.003"	n/a	n/a	4	02-03-00
Span / Depth	3.7	n/a	n/a		00-00-00

Bearing Supports		Dim.(LxW) Demand		De mand/ Resistance Support	De mand/ Resistance Member	Material	
B0	Wall/Plate	5-1/2" x 1-3/4"	876 lbs	21.3%	7.5%	Un specified	
B1	Post	3-1/2" x 1-3/4"	1,065 lbs	26.8%	14.3%	Un specified	

Notes

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume unbraced length of Top: 03-10-08, Bottom: 03-10-08. Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA O86. CONFORMS TO OBC 2012

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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Products L.L.C.



DWO NO . TAM 4205217 STRUCTURAL COMPONENT ONLY



Triple 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP 1st Floor\Flush Beams\B7(i2188)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

... August 21, 2017 08:10:57

Build 5033

Job Name:

Address: City, Province, Postal Code:WATERDOWN,

Customer:

Code reports:

CCMC 12472-R

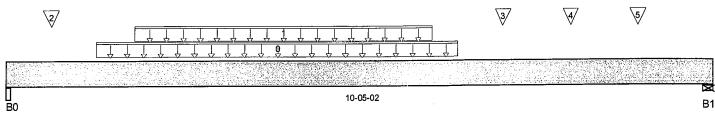
File Name: ROSEWOOD 1A EL-1.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B7(i2188)

Specifier:

Designer: Company:

Misc:



Reaction Summary (Down / Uplift) (lbs)								
Bearing	Live	De ad	Snow	Wind				
B0, 4-1/2"	2,258 / 0	1,222 / 0						
B1, 5-1/2"	2,043 / 0	1,116/0						

10	ad Summary					Live	Dead	Snow	Wind	Trib.
	g Description	Load Type	Re	f. Start	En d	1.00	0.65	1.00	1.15	
0	Smoothed Load	Unf. Lin. (lb/ft)	L	01-03-14	06-07-14	338	168			n/a
1	Us er Load	Unf. Lin. (lb/ft)	L	01-10-10	06-03-02	240	120			n/a
2	J1(i2356)	Conc. Pt. (lbs)	L	00-07-14	00-07-14	299	150			n/a
3	J1(i2234)	Conc. Pt. (lbs)	L	07-03-14	07-03-14	394	197			n/a
4	J1 (i2241)	Conc. Pt. (lbs)	L	08-03-14	08-03-14	338	169			n/a
5	J1 (i2238)	Conc. Pt. (lbs)	L	09-03-14	09-03-14	387	194			n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	12,710 ft-lbs	60,415 ft-lbs	21%	1	04-07-14
End Shear	4,425 lbs	21,696 lbs	20.4%	1	01-04-06
Total Load Defl.	L/999 (0.102")	n/a	n/a	4	05-01-14
Live Load Defl.	L/999 (0.066")	n/a	n/a	5	05-01-14
Max Defl.	0.102"	n/a	n/a	4	05-01-14
Span / Depth	9.8	n/a	n/a		00-00-00

Bearing Supports		Dim.(LxW) Demand		Resistance Support	Resistance Member	Material	
B0	Beam	4-1/2" x 5-1/4"	4,914 lbs	48.7%	17%	Unspecified	
B1	Wall/Plate	5-1/2" x 5-1/4"	4,460 lbs	36.2%	12.7%	Unspecified	

Notes

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA CONFORMS TO OBC 2012 O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

DWG NO. TAN 4285317 STRUCTURAL COMPONENT ONLY



Triple 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP 1st Floor\Flush Beams\B7(i2188)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

"August 21, 2017 08:10:57

BC CALC® Design Report

CCMC 12472-R

Build 5033 Job Name:

Address: City, Province, Postal Code: WATERDOWN,

Customer:

Code reports:

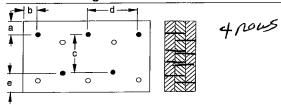
File Name: ROSEWOOD 1A EL-1.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B7(i2188

Specifier: Designer:

Company. Misc:

Connection Diagram



a minimum = #" c = @-7/8" d= 2 6 b minimum = 3" e minimum = 3"

Calculated Side Load = 655.7 lb/ft

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record. Nailing schedule applies to both sides of the member.

Connectors are: 16d Nails
3½" ARDOX SPIRAL

Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered w ood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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STRUCTURAL COMPONENT ONLY



Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B8(i2247)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 21, 2017 08:10:57

BC CALC® Design Report



BO OALOW Besign Reper

Build 5033 Job Name:

Address:
City, Province, Postal Code:WATERDOWN,

Customer:

Code reports:

CCMC 12472-R

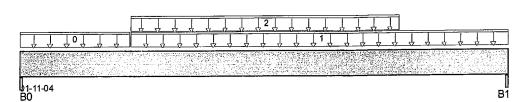
File Name: ROSEWOOD 1A EL-1.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\88(i2247)

Specifier:

Designer:

Company. Misc:



Total Horizontal Product Length = 01-11-04

Reaction Summary (Down / Uplift) (lbs)									
Bearing	Live	De ad	Snow	Wind					
B0, 5-1/4"	50 / 0	94 / 0	61 / 0						
B1, 5-1/4"	47 / 0	92 / 0	57 / 0						

۱.	ad Summary					Live	Dead	Snow	Wind	Trib.
	g Description	Load Type	Ref.	Start	En d	1.00	0.65	1.00	1.15	
0	FC2 Floor Material	Unf. Lin. (lb/ft)	L (00-00-00	00-05-04	56	41	69		n/a
1	FC2 Floor Material	Unf. Lin. (lb/ft)	L (00-05-04	01-11-04	48	36	59		n/a
2	FC2 Floor Material	Unf. Lin. (lb/ft)	L (00-05-04	01-06-00		85			n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location	
Pos. Moment	33 ft-lbs	25,173 ft-lbs	0.1%	0	00-11-10	
End Shear	85 lbs	9,401 lbs	0.9%	0	01-05-02	
Span / Depth	1.2	n/a	n/a		00-00-00	

				De mand/	Demand/	
				Resistance	Resistance	
Bearing Supports		Dim.(LxW)	Demand	Support	Member	Material
B0	Beam	5-1/4" x 3-1/2"	235 lbs	3%	1%	Unspecified
B1	Beam	5-1/4" x 3-1/2"	224 lbs	2.8%	1%	Unspecified

Notes

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA 086. CONFORMS TO OBC 2012

Unbalanced snow loads determined from building geometry were used in selected product's verification.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

S. KATSOULAKOS S

DWG NG. TAM 4285417

STRUCTURAL COMPONENT ONLY



Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B8(i2247)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 21, 2017 08:10:57

BC CALC® Design Report

*

Build 5033

Job Name:

Address:

City, Province, Postal Code:WATERDOWN,

Customer:

Code reports:

CCMC 12472-R

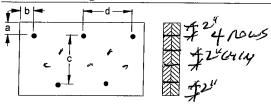
File Name: ROSEWOOD 1A EL-1 mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\88(i2247

Specifier: Designer: Company.

Misc:

Connection Diagram



a minimum = 2" b minimum = 3" c=7-7/8" d= 20 6

Member has no side loads.

Connectors are: 16d C. Nails
3½" ARDOX SPIRAL

Disclosure

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DWG NO.TAM 4205417 STRUGTURAL COMPONENT ONLY



Triple 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B3(i2507)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 21, 2017 08:11:29

Build 5033

Job Name: Address:

City, Province, Postal Code: WATERDOWN,

Customer:

Code reports:

CCMC 12472-R

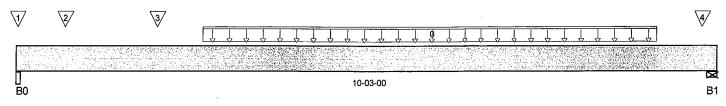
File Name: ROSEWOOD 1A EL-1.mmdl

Description: Designs\Flush Beams\Basment\Flush Beams\B3(i2507)

Specifier:

Designer: Company:

Misc:



Total Horizontal Product Length = 10-03-00

Reaction Summary (Down / Uplift) (lbs)								
Be aring	Live	De ad	Snow	Wind				
B0, 2-5/8"	2,602/0	1,428 / 0						
B1, 5-1/2"	5.072 / 0	3,808/0						

Load Summary Tag Description						Live	Dead	Snow	Wind	Trib.
		Load Type Ref. Start En		En d	1.00	0.65	1.00	1.15		
0	Smoothed Load	Unf. Lin. (lb/ft)	L	02-08-08	09-04-08	595	298			n/a
1	7(i352)	Conc. Pt. (lbs)	L	00-00-04	00-00-04	149	110			n/a
2	J1 (i2526)	Conc. Pt. (lbs)	L	00-08-08	00-08-08	362	181			n/a
3	J1 (i2539)	Conc. Pt. (lbs)	L	02-00-08	02-00-08	692	346			n/a
4	4(i345)	Conc. Pt. (lbs)	L	10-00-04	10-00-04	2,503	2,428			n/a

	Factored	Factored	Demand /	Load	Location
Controls Summary	Dem and	Resistance	Resistance	Case	•
Pos. Moment	14,755 ft-lbs	60,415 ft-lbs	24.4%	1	04-08-08
End Shear	5,545 lbs	21,696 ibs	25.6%	1	08-09-10
Total Load Defl.	L/999 (0.119")	n/a	n/a	4	05-00-08
Live Load Defl.	L/999 (0.078")	n/a	n/a	5	05-00-08
Max Defl.	0.119"	n/a	n/a	4	05-00-08
Span / Depth	9.8	n/a	n/a		00-00-00

Beari	ng Supports	Dim.(L x W)	Demand	Resistance Support	Resistance Member	Material
B0	Beam	2-5/8" x 5-1/4"	5,688 lbs	77.3%	33.8%	Unspecified
B1	Wall/Plate	5-1/2" x 5-1/4"	12,367 lbs	80.2%	35.1%	Unspecified

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA

CONFORMS TO OBC 2012

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9



DWO NO . YAM 4285-17 STRUCTURAL COMPONENT ONLY



Triple 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B3(i2507)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 21, 2017 08:11:29

BC CALC® Design Report



Build 5033 Job Name:

Address:

City, Province, Postal Code:WATERDOWN,

Customer:

Code reports:

CCMC 12472-R

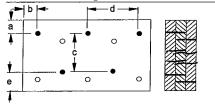
File Name: ROSEWOOD 1A EL-1.mmdl

Description: Designs\Flush Beams\Basment\Flush Beams\B3(i250'

Specifier: Designer:

Company: Misc:

Connection Diagram



a minimum =**#**" b minimum = 3"

c=6-7/8" d= 8 4 11 e minimum = 2

Calculated Side Load = 1,041.1 lb/ft

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record. Nailing schedule applies to both sides of the member.

Connectors are: 16d Small Nails
3½" ARDOX SPIRAL

Disclosure

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POLINCE OF ONTO

DWG NO. TAN 42855 STRUCTURAL COMPONENT ONLY



Live Load = 40 psf, Dead Load = 15 psf Simple Spans, L/480 Deflection Limit 3/4" OSB G&N Sheathing







			В	are			1/2" Gyp	sum Ceiling		
Depth	Series		On Cent	re Spacing			On Centre Spacing			
		12"	16"	19.2"	24"	12"	16"	/ 19.2"	24"	
	NI-20	15'-10"	15'-0"	14'-5"	13'-5"	16'-4"	15'-5"	14'-6"	13'-5"	
	NI-40x	17'-0"	16'-0"	15'-5"	14'-9"	17'-5"	16'-5"	15'-10"	15'-2"	
9-1/2"	NI-60	17'-2"	16'-2"	15'-7"	14'-11"	17'-6"	16'-7"	15'-11"	15'-3"	
	NI-70	18'-0"	16'-11"	16'-3"	15'-7"	18'-5"	17' - 3"	16'-7"	15'-11"	
	NI-80	18'-3"	17'-1"	16'-5"	15'-9"	18'-8"	17'-5"	16'-9"	16'-1"	
	NI-20	17'-10"	16'-10"	16'-2"	15'-6"	18'-6"	17'-4"	16'-9"	16'-1"	
	NI-40x	19'-4"	17'-11"	17'-3"	16'-6"	19'-11"	18'-6"	17'-9"	17'-0"	
11-7/8"	NI-60	19'-7"	18'-2"	17'-5"	16'-9"	20'-2"	18'-9"	17'-11"	17'-2"	
11-//0	NI-70	20'-9"	19'-2"	18'-3"	17'-5"	21'-4"	19'-9"	18'-10"	17'-10"	
	NI-80	21'-1"	19'-5"	18'-6"	17'-7"	21'-7"	20'-0"	19'-0"	18'-0"	
	NI-90x	21'-8"	20'-0"	19'-1"	18'-0"	22'-2"	20'-6"	19'-6"	18'-6"	
	NI-40x	21'-5"	19'-10"	18'-11"	17'-11"	22'-1"	20'-6"	19'-7"	18'-7"	
	NI-60	21'-10"	20'-2"	19'-3"	18'-2"	22'-5"	20'-10"	19'-11"	18'-10"	
14"	NI-70	23'-0"	21'-3"	20'-3"	19'-2"	23'-8"	21'-11"	20'-10"	19'-9"	
	NI-80	23'-5"	21'-7"	20'-7"	19'-5"	24'-0"	22' - 3"	21'-2"	20'-0"	
	NI-90x	24'-1"	22'-3"	21'-2"	20'-0"	24'-8"	22'-10"	21'-9"	20'-7"	
	NI-60	23'-9"	22'-0"	20'-11"	19'-10"	24'-6"	22'-9"	21'-8"	20'-6"	
16"	NI-70	25'-1"	23'-2"	22'-0"	20'-10"	25'-9"	23'-10"	22'-9"	21'-6"	
10	NI-80	25 '- 6"	23'-6"	22'-4"	21'-2"	26'-1"	24'-2"	23'-1"	21'-10"	
	NI-90x	26'-4"	24'-3"	23'-1"	21'-10"	26'-11"	24'-11"	23'-8"	22'-5"	

			Mid-Spa	n Blocking		Mid-9	pan Blocking ar	nd 1/2" Gypsum	Ceiling
Depth	Series		On Cent	re Spacing			On Cent	re Spacing	******
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	16'-10"	15'-5"	14'-6"	13'-5"	16'-10"	15'-5"	14'-6"	13'-5"
	NI-40x	18'-8"	17'-2"	16'-3"	15'-2"	18'-10"	17'-2"	16'-3"	15'-2"
9-1/2"	NI-60	18'-11"	17'-6"	16'-6"	15'-5"	19'-2"	17'-6"	16'-6"	15'-5"
	NI-70	20'-0"	18'-7"	17'-9"	16'-7"	20'-5"	18'-11"	17'-10"	16'-7"
	NI-80	20'-3"	18'-10"	17'-11"	16'-10"	20'-8"	19'-3"	18'-2"	16'-10"
	NI-20	20'-1"	18'-5"	17'-5"	16'-2"	20'-1"	18'-5"	17'-5"	16'-2"
	NI-40x	21'-10"	20'-4"	19'-4"	17:-8"	22'-5"	20'-6"	19'-4"	17'-8"
11 7/0"	NI-60	22'-1"	20'-7"	19'-7"	18'-4"	22'-8"	20'-10"	19'-8"	18'-4"
11-7/8"	NI-70	23'-4"	21'-8"	20'-8"	19'-7"	23'-10"	22'-3"	21'-2"	19'-9"
	NI-80	23'-7"	21'-11"	20'-11"	19'-9"	24'-1"	22'-6"	21'-5"	20'-0"
	NI-90x	24'-3"	22'-6"	21'-6"	20'-4"	24'-8"	23'-0"	22'-0"	20'-9"
	NI-40x	24'-5"	22'-9"	21'-8"	19'-5"	25'-1"	23'-2"	21'-9"	19'-5"
	NI-60	24'-10"	23'-1"	22'-0"	20'-10"	25'-6"	23'-8"	22'-4"	20'-10"
14"	NI-70	26'-1"	24 '- 3"	23'-2"	21'-10"	26'-8"	24'-11"	23'-9"	22'-4"
	NI-80	26'-6"	24'-7"	23'-5"	22'-2"	27'-1"	25'-3"	24'-1"	22'-9"
	NI-90x	27'-3"	25'-4"	24'-1"	22'-9"	27'-9"	25'-11"	24'-8"	23'-4"
	NI-60	27'-3"	25'-5"	24'-2"	22'-10"	28'-0"	26'-2"	24'-9"	23'-1"
16"	NI-70	28'-8"	26'-8"	25'-4"	23'-11"	29'-3"	27'-4"	26'-1"	24'-8"
10	NI-80 .	29 '- 1"	27'-0"	25'-9"	24'-4"	29'-8"	27'-9"	26 '- 5"	25'-0"
	NI-90x	29'-11"	27'-10"	26'-6"	25'-0"	30'-6"	28'-5"	27'-2"	25'-8"

^{1.} Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 15 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of L/480 and a total load deflection limit of L/240.

Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 3/4 inch for a joist spacing of 24 inches or less. The composite floor may include 1/2 inch gypsum ceiling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attached to joists.
 Minimum bearing length shall be 1-3/4 inches for the end bearings.

^{4.} Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.

^{5.} This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA O86-09, NBC 2010, and OBC 2012.

^{6.} Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.



Live Load = 40 psf, Dead Load = 15 psf Simple Spans, L/480 Deflection Limit 5/8" OSB G&N Sheathing







			В.	are		1	1/2" Gyp	sum Ceiling	
Depth	Series		On Cent	re Spacing			On Cent	re Spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	15'-1"	14'-2"	13'-9"	N/A	15'-7"	14'-8"	14'-2"	N/A
	NI-40x	16'-1"	15'-2"	14'-8"	N/A	16'-7"	1 5'-7"	15'-1"	N/A
9-1/2"	NI-60	16'-3"	15'-4"	14'-10"	N/A	16'-8"	15'-9"	15'- 3"	N/A
	NI-70	17'-1"	16'-1"	15'-6"	N/A	17'-5"	16'-5"	15'-10"	N/A
	NI-80	17'-3"	16'-3"	15'-8"	N/A	17'-8"	16'-7"	16'-0"	N/A
	NI-20	16'-11"	16'-0"	15'-5"	N/A	17'-6"	16'-6"	16'-0"	N/A
	NI-40x	18'-1"	17'-0"	16'-5"	N/A	18'-9"	17'-6"	16'-11"	N/A
11 7/0"	NI-60	18 '- 4"	17'-3"	16'-7"	N/A	19'-0"	17'-8"	17'-1"	N/A
11-7/8"	NI-70	19'-6"	18'-0"	17'-4"	N/A	20'-1"	18'-7"	17'-9"	N/A
	NI-80	19'-9"	18'-3"	17'-6"	N/A	20'-4"	18'-10"	17'-11"	N/A
	NI-90x	20'-4"	18'-9"	17'-11"	N/A	20'-10"	19'-3"	18'-5"	N/A
	NI-40x	20'-1"	18'-7"	17'-10"	N/A	20'-10"	19'-4"	18'-6"	N/A
	NI-60	20'-5"	18'-11"	18'-1"	N/A	21'-2"	19'-7"	18' - 9"	N/A
14"	NI-70	21'-7"	20'-0"	19'-1"	N/A	22'-3"	20'-7"	19'-8"	N/A
	NI-80	21'-11"	20'-3"	19'-4"	N/A	22'-7"	20'-11"	20'-0"	N/A
	NI-90x	22'-7"	20'-11"	19'-11"	N/A	23'-3"	21'-6"	20'-6"	N/A
	NI-60	22'-3"	20'-8"	19'-9"	N/A	23'-1"	21'-5"	20'-6"	N/A
16"	NI-70	23'-6"	2 1'- 9"	20'-9"	N/A	24'-3"	22'-5"	21'-5"	N/A
70	NI-80	23'-11"	22'-1"	21'-1"	N/A	24'-8"	22'-10"	21'-9"	N/A
	NI-90x	24'-8"	22'-9"	2 1'- 9"	N/A	25'-4"	23'-5"	22'-4"	N/A

			Mid-Spa	n Blocking		Mid-S	pan Blocking ar	nd 1/2" Gypsum	Ceiling
Depth	Series		On Cent	re Spacing		On Centre Spacing			
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	16'-8"	15'-3"	14'-5"	N/A	16'-8"	15'-3"	14'-5"	N/A
	NI-40x	17'-11"	16'-11"	16'-1"	N/A	18'-5"	17'-1"	16'-1"	N/A
9-1/2"	NI-60	18'-2"	17'-1"	16'-4"	N/A	18'-7"	17'-4"	16'-4"	N/A
	NI-70	19'-2"	17'-10"	17'-2"	N/A	19'-7"	18'-3"	17'-7"	N/A
	NI-80	19'-5"	18'-0"	17'-4"	N/A	19'-10"	18'-5"	17'-8"	N/A
	NI-20	19'-6"	18'-1"	17'-3"	N/A	19'-11"	18'-3"	17'-3"	N/A
	NI-40x	21'-0"	19'-6"	18'-8"	N/A	21'-7"	20'-2"	19'-2"	N/A
11 7/01	NI-60	21'-4"	19'-9"	18'-11"	N/A	21'-11"	20'-4"	19'-6"	N/A
11-7/8"	NI-70	22'-6"	20'-10"	19'-11"	N/A	23'-0"	21'-5"	20'-5"	N/A
	NI-80	22'-9"	21'-1"	20'-1"	N/A	23'-3"	21'-7"	20'-8"	N/A
	NI-90x	23'-4"	21'-8"	20'-8"	N/A	23'-10"	22'-2"	21'-2"	N/A
	NI-40x	23'-7"	21'-11"	20'-11"	N/A	24'-3"	22'-7"	21'-7"	N/A
	NI-60	24'-0"	22'-3"	21'-3"	N/A	24'-8"	22'-11"	21'-11"	N/A
14"	NI-70	25'-3"	23'-4"	22'-3"	N/A	25'-10"	24'-0"	22'-11"	N/A
	NI-80	25'-7"	23'-8"	22' - 7"	N/A	26'-2"	24'-4"	23'-2"	N/A
	NI-90x	26'-4"	24'-4"	23'-3"	N/A	26'-10"	24'-11"	23'-9"	N/A
	NI-60	26'-5"	24'-6"	23'-4"	N/A	27'-2"	25'-3"	24'-2"	N/A
16"	NI-70	27'-9"	25'-8"	24'-6"	N/A	28'-5"	26'-5"	25'-2"	N/A
10	NI-80	28'-2"	26'-1"	24'-10"	N/A	28'-10"	26' - 9"	25'-6"	N/A
	NI-90x	29'-0"	26'-10"	25'-7"	N/A	29'-7"	27'-5"	26'-2"	N/A

^{1.} Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 15 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of L/480 and a total load deflection limit of L/240.

^{2.} Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 5/8 inch for a joist spacing of 19.2 inches or less. The composite floor may include 1/2 inch gypsum ceiling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attached to joists.

^{3.} Minimum bearing length shall be 1-3/4 inches for the end bearings.

^{4.} Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.

^{5.} This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA O86-09, NBC 2010, and OBC 2012.

^{6.} Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.



Live Load = 40 psf, Dead Load = 30 psf Simple Spans, L/480 Deflection Limit 5/8" OSB G&N Sheathing







			B	are			1/2" Gyp	sum Ceiling		
Depth	Series	-	On Cent	re Spacing			On Centre Spacing			
		12"	16"	19.2"	24"	12"	16"	19.2"	24"	
	NI-20	15'-1"	14'-1"	13'-3"	N/A	15'-7"	14'-1"	13'-3"	N/A	
	NI-40x	16'-1"	15'-2"	14'-8"	N/A	16'-7"	15'-7"	15'-1"	N/A	
9-1/2"	Ni-60	16'-3"	15'-4"	14'-10"	N/A	16'-8"	15'-9"	15'-3"	N/A	
	NI-70	17'-1"	16'-1"	15'-6"	N/A	17'-5"	16'-5"	15'-10"	N/A	
	NI-80	17'-3"	16'-3"	15'-8"	N/A	17'-8"	16'-7"	16'-0"	N/A	
	NI-20	16'-11"	16'-0"	15'-5"	N/A	17'-6"	16'-6"	16'-0"	N/A	
	NI-40x	18'-1"	17'-0"	16'-5"	N/A	18'-9"	17'-6"	16'-11"	N/A	
11-7/8"	NI-60	18'-4"	17'-3"	16'-7"	N/A	19'-0"	17'-8"	17'-1"	N/A	
11-//0	NI-70	19'-6"	18'-0"	17'-4"	N/A	20'-1"	18'-7"	17'-9"	N/A	
	NI-80	19'-9"	18'-3"	17'-6"	N/A	20'-4"	18'-10"	17'-11"	N/A	
	NI-90x	20'-4"	18'-9"	17'-11"	N/A	20'-10"	19'-3"	18' - 5"	N/A	
	NI-40x	20'-1"	18'-7"	17'-10"	N/A	20'-10"	19'-4"	18'-6"	N/A	
	NI-60	20' - 5"	18'-11"	18'-1"	N/A	21'-2"	19'-7"	18'-9"	N/A	
14"	NI-70	21'-7"	20'-0"	19'-1"	N/A	22'-3"	20'-7"	19'-8"	N/A	
	NI-80	21'-11"	20'-3"	19'-4"	N/A	22'-7"	20'-11"	20'-0"	N/A	
	NI-90x	22'-7"	20'-11"	19'-11"	N/A	23'-3"	21'-6"	20'-6"	N/A	
	NI-60	22'-3"	20'-8"	19'-9"	N/A	23'-1"	21'-5"	20'-6"	N/A	
16"	NI-70	23'-6"	21'-9"	20'-9"	N/A	24'-3"	22'-5"	21'-5"	N/A	
10	NI-80	23'-11"	22'-1"	21'-1"	N/A	24'-8"	22'-10"	21'-9"	N/A	
	NI-90x	24'-8"	22'-9"	21'-9"	N/A	25'-4"	23'-5"	22'-4"	N/A	

			Mid-Spa	n Blocking		Mid-9	pan Blocking ar	nd 1/2" Gypsum	Ceiling
Depth	Series		On Centre Spacing On Centre Spacing						
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	15'-7"	14'-1"	13'-3"	N/A	15'-7"	14'-1"	13'-3"	N/A
	NI-40x	17'-9"	16'-1"	15'-1"	N/A	17'-9"	16'-1"	15'-1"	N/A
9-1/2"	NI-60	18'-1"	16'-4"	15'-4"	N/A	18'-1"	16'-4"	15'-4"	N/A
	NI-70	19'-2"	17'-10"	16'-9"	N/A	19'-7"	17'-10"	16'-9"	N/A
	NI-80	19'-5"	18'-0"	17'-1"	N/A	19'-10"	18'-3"	17'-1"	N/A
	NI-20	18'-9"	17'-0"	16'-0"	N/A	18'-9"	17'-0"	16'-0"	N/A
	NI-40x	21'-0"	19'-3"	17'-9"	N/A	21'-3"	19'-3"	17'-9"	N/A
11-7/8"	NI-60	21'-4"	19'-8"	18'-5"	N/A	21'-8"	19'-8"	18'-5"	N/A
11-1/0	NI-70	22'-6"	20'-10"	19'-11"	N/A	23'-0"	21'-4"	20'-0"	N/A
	NI-80	22'-9"	21'-1"	20'-1"	N/A	23'-3"	21'-7"	20'-5"	N/A
	NI-90x	23'-4"	21'-8"	20'-8"	N/A	23'-10"	22'-2"	21'-2"	N/A
	NI-40x	23'-7"	21'-5"	19'-6"	N/A	24'-1"	21'-5"	19'-6"	N/A
	NI-60	24'-0"	22'-3"	21'-0"	N/A	24'-8"	22'-5"	21'-0"	N/A
14"	NI-70	25 '- 3"	23'-4"	22'-3"	N/A	25'-10"	24'-0"	22'-9"	N/A
	NI-80	25'~7"	23'-8"	22'-7"	N/A	26'-2"	24'-4"	23'-2"	N/A
•	NI-90x	26'-4"	24'-4"	23'-3"	N/A	26'-10"	24'-11"	23'-9"	N/A
	NI-60	26'-5"	24'-6"	23'-4"	N/A	27'-2"	24'-10"	23'-4"	N/A
16"	NI-70	27'-9"	25'-8"	24'-6"	N/A	28'-5"	26'-5"	25'-2"	N/A
10	NI-80	28'-2"	26'-1"	24'-10"	N/A	28'-10"	26'-9"	25'-6"	N/A
	NI-90x	29'-0"	26'-10"	25'-7"	N/A	29'-7"	27'-5"	26'-2"	N/A

^{1.} Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 30 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of L/480 and a total load deflection limit of L/240.

^{2.} Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 5/8 inch for a joist spacing of 19.2 inches or less. The composite floor may include 1/2 inch gypsum ceiling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attached to joists.

^{3.} Minimum bearing length shall be 1-3/4 inches for the end bearings.

^{4.} Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.

^{5.} This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA O86-09, NBC 2010, and OBC 2012.

^{6.} Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.



Live Load = 40 psf, Dead Load = 30 psf Simple Spans, L/480 Deflection Limit 3/4" OSB G&N Sheathing







			В	are			1/2" Gyp	sum Ceiling	
Depth	Series		On Cent	re Spacing			On Cent	re Spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	15'-7"	14'-2"	13'-4"	12'-4"	15'-7"	14'-2"	13'-4"	12'-4"
	NI-40x	17'-0"	16'-0"	15'-1"	13'-11"	17'-5"	16'-1"	15'-1"	13'-11"
9-1/2"	NI-60	17'-2"	16'-2"	1 5' - 5"	14'-3"	17'-6"	16'-5"	15'-5"	14'-3"
	NI-70	18'-0"	16'-11"	16'-3"	15'-6"	18'-5"	17'-3"	16'-7"	15'-6"
	NI-80	18'-3"	17'-1"	16'-5"	15'-9"	18'-8"	17'-5"	16'-9"	15'-10"
	NI-20	17'-10"	16'-10"	16'-0"	14'-10"	18'-6"	17'-1"	16'-0"	14'-10"
	NI-40x	19'-4"	17'-11"	17'-3"	15'-10"	19'-11"	18'-6"	17'-9"	15'-10"
11-7/8"	NI-60	19'-7"	18'-2"	17'-5"	16'-9"	20'-2"	18'-9"	17'-11"	17'-1"
11-7/6	NI-70	20'-9"	19'-2"	18'-3"	17'-5"	21'-4"	19'-9"	18'-10"	17'-10"
	NI-80	21'-1"	19'-5"	18'-6"	17'-7"	21'-7"	20'-0"	19'-0"	18'-0"
	NI-90x	21'-8"	20'-0"	19'-1"	18'-0"	22'-2"	20'-6"	19'-6"	18'-6"
	NI-40x	21'-5"	19'-10"	18'-11"	17'-5"	22'-1"	20'-6"	19'-6"	17'-5"
	NI-60	21'-10"	20'-2"	19'-3"	18'-2"	22'-5"	20'-10"	19'-11"	18'-10"
14"	NI-70	23'-0"	21'-3"	20'-3"	19'-2"	23'-8"	21'-11"	20'-10"	19'-9"
	NI-80	23 '- 5"	21'-7"	20'-7"	19'-5"	24'-0"	22'-3"	21'-2"	20'-0"
	N!-90x	24'-1"	22'-3"	21'-2"	20'-0"	24'-8"	22'-10"	21'-9"	20'-7"
	NI-60	23'-9"	22'-0"	20'-11"	19'-10"	24'-6"	22'-9"	21'-8"	20'-6"
16"	NI-70	25'-1"	23'-2"	22'-0"	20'-10"	25'-9"	23'-10"	22'-9"	21'-6"
10	NI-80	25' - 6"	23'-6"	22'-4"	21'-2"	26'-1"	24'-2"	23'-1"	21'-10"
	NI-90x	26'-4"	24' - 3"	23'-1"	21'-10"	26'-11"	24'-11"	23'-8"	22'-5"

			Mid-Spa	n Blocking		Mid-S	Span Blocking a	nd 1/2" Gypsum	Ceiling
Depth	Series		On Cent	re Spacing]	On Cent	re Spacing	***************************************
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	15'-7"	14'-2"	13'-4"	12'-4"	15'-7"	14'-2"	13'-4"	12'-4"
	NI-40x	17'-9"	16'-1"	15'-1"	13'-11"	17'-9"	16'-1"	15'-1"	13'-11"
9-1/2"	NI-60	18'-1"	16'-5"	15'-5"	14'-3"	18'-1"	16'-5"	15'-5"	14'-3"
	NI-70	19'-10"	17'-11"	16'-9"	15'-6"	19'-10"	17'-11"	16'-9"	15'-6"
	NI-80	20'-2"	18'-3"	17'-1"	15'-10"	20'-2"	18'-3"	17'-1"	15'-10"
	NI-20	18'-10"	17'-1"	16'-0"	14'-10"	18'-10"	17'-1"	16'-0"	14'-10"
	NI-40x	21'-3"	19'-3"	17'-9"	15'-10"	21'-3"	19'-3"	17'-9"	15'-10"
11-7/8"	NI-60	21'- 9 "	19' - 8"	18'-5"	17'-1"	21'-9"	19'-8"	18'-5"	17'-1"
11-7/6	NI-70	23'-4"	21'-5"	20'-1"	18'-6"	23'-8"	21'-5"	20'-1"	18'-6"
	NI-80	23'-7"	21'-10"	20'-5"	18'-11"	24'-1"	21'-10"	20'-5"	18'-11"
	NI-90x	24'-3"	22'-6"	21'-3"	19'-7"	24'-8"	22'-7"	21'-3"	19'-7"
	NI-40x	24'-2"	21'-5"	19'-6"	17'-5"	24'-2"	21'-5"	19'-6"	17'-5"
	NI-60	24'-9"	22'-5"	21'-0"	19'-6"	24'-9"	22'-5"	21'-0"	19'-6"
14"	NI-70	26'-1"	24'-3"	22'-9"	21'-0"	26'-8"	24'-3"	22'-9"	21'-0"
	NI-80	26'-6"	24'-7"	23'-3"	21'-6"	27'-1"	24'-10"	23'-3"	21'-6"
	NI-90x	27'-3"	25'-4"	24'-1"	22'-4"	27'-9"	25'-10"	24'-3"	22'-4"
	NI-60	27'-3"	24'-11"	23'-5"	21'-7"	27'-6"	24'-11"	23'-5"	21'-7"
16"	NI-70	28'-8"	26'-8"	25 '- 3"	23'-4"	29'-3"	26'-11"	25'-3"	23'-4"
10	NI-80	29'-1"	27'-0"	25'-9"	23'-10"	29'-8"	27'-6"	25'-10"	23'-10"
	NI-90x	29'-11"	27'-10"	26'-6"	24'-10"	30'-6"	28'-5"	26'-11"	24'-10"

^{1.} Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 30 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of L/480 and a total load deflection limit of L/240.

Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 3/4 inch for a joist spacing of 24 inches or less. The composite floor may include 1/2 inch gypsum ceiling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attached to joists.
 Minimum bearing length shall be 1-3/4 inches for the end bearings.

^{4.} Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.

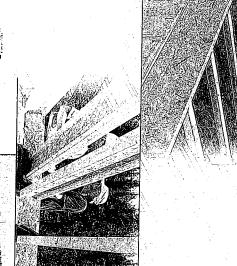
^{5.} This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA O86-09, NBC 2010, and OBC 2012.

^{6.} Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.

ENGINEERED WOOD

NSTALIATION GUIDE

FOR RESIDENTIAL FLOORS







Distributed by:

N-C301 / November 2014

SAFETY AND CONSTRUCTION PRECAUTIONS

WARNING



braced and sheathed.

Avoid Accidents by Following these Important Guidelines:

Brace and nail each I-joist as it is installed, using hangers, blocking panels, rim board, and/or cross-bridging at joist ends. When I-joists are applied continuous over interior supports and a load-bearing wall is planned at that location,

i-joists are not stable until completely installed, and will not carry any load until fully

Do not walk on I-joists until fully fastened and braced, or serious injuries can result.



concentrated loads from Once sheathed, do not over-stress I-joist with unsheathed I-joists. Never stack building building materials. materials over





• Iemporary bracing or struts must be 1x4 inch minimum, at least 8 feet long and spaced no more than 8 feet on centre, and must be secured with a minimum. to prevent l-joist rollover or buckling. minimum of two 2-1/2" nails fastened to the top surface of each Ligist. Nail the bracing to a lateral restraint at the end of each bay. Lap ends of adjoining

When the building is completed, the floor sheathing will provide lateral support for the top flanges of the I-joists. Until this sheathing is applied,

blocking will be required at the interior support.

temporary bracing, often called struts, or temporary sheathing must be applied

 Or, sheathing (temporary or permanent) can be nailed to the top flange of the first 4 feet of I-joists at the end of the bay. bracing over at least two 1-joists.

4. Install and fully nail permanent sheathing to each Lipist before placing loads 3. For cantilevered Lioists, brace top and bottom flanges, and brace ends with closure panets, rim board, or cross-bridging.

Never install a damaged I-joist. on the floor system. Then, stack building materials over beams or walls only,

Improper storage or installation, failure to follow applicable building codes, failure to follow span ratings for Nordic I-joists, failure to follow allowable hole sizes and locations, or failure to use web stiffeners when required can result in serious accidents. Follow these installation guidelines carefully,

STORAGE AND HANDLING GUIDELINES

Bundle wrap can be slippery when wet. Avoid walking on wrapped

2. Store, stack, and handle 1-joists vertically and level only.

Always stack and handle 1-joists in the upright position only.

4. Do not store I-joists in direct contact with the ground and/or flatwise

Protect I-joists from weather, and use spacers to separate bundles.

Bundled units should be kept intact until time of installation.

7. When handling I-joists with a crane on the job site, take a few

simple precautions to prevent damage to the I-joists and injury to your work crew.

■ Pick I-joists in bundles as shipped by the supplier.

■ Orient the bundles so that the webs of the I-joists are vertical.

■Pick the bundles at the 5th points, using a spreader bar if necessary

8. Do not handle Ljoists in a horizontal orientation.

9. NEVER USE OR TRY TO REPAIR A DAMAGED 1-JOIST.







MAXIMUM FLOOR SPANS

- . Maximum **clear** spans applicable to simple-span or or more of the adjacent span. multiple-span residential floor construction with a design live load of 40 psf and dead load of 15 psf. The ultimate For multiple-span applications, the end spans shall be 40% tor floor vibration and a live load deflection limit of L/480 limit states are based on the factored loads of 1.50L + .25D. The serviceability limit states include the consideration
- Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 5/8 inch for a joist spacing of 19.2 inches or less, or 3/4 inch for joist spacing of 24 inches. Adhesive shall meet the requirements given in CGBS-71.26 assumed. Increased spans may be achieved with the used of gypsum and/or a row of blocking at mid-span. Standard. No concrete topping or bridging element was
- Minimum bearing length shall be 1.3/4 inches for the end bearings, and 3-1/2 inches for the intermediate bearings.
- Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.
- This span chart is based on uniform loads. For applications with other than uniform loads, an engineering analysis may be required based on the use of the design properties.
- 6. Tables are based on Limit States Design per CAN/CSA O86-09 Standard, and NBC 2010.
- 7. SI units conversion: 1 inch = 25.4 mm 1 foot = 0.305 m

MAXIMUM FLOOR SPANS FOR NORDIC I-JOISTS

SIMPLE AND MULTIPLE SPANS

				Joist J Depth S
				Joist Series
22.3 23.4 23.11 24.5 24.5	28 20 5 21 7 21 11 21 11 22 5	1976 1976 2079	77.6	12"
22.22.48 22.22.48 22.22.48 22.23.48	18:11 20:0 20:0 20:3 20:8	16.0° 17.0° 17.3° 18.3° 18.3° 18.3°	154 154 31,4	Simple On centre 16"
20-9 20-9 21-5 21-5	17.10 18:1 19:1 19:4 19:9	17.10 17.10	13:9: 14:8: 14:10: 15:6:	spans spacing 19.2
20-10 20-10 21-20 21-30	17.41 18.22 19.22 20.13 20.13	0.00 (0.00 (0.00 (0.00 (0.00)	137.57 14.107 14.101 15.77	24"
	22-72 23-72 23-70 24-73 25-74	100 H 100 H	25.91 27.01 27.01 27.01 28.91	12"
0.000 10.000 10.000	ANGRES September			Multiple On centre 16"
20000 20000			8 6 0 3 4 0 1 0 0	e spans e spacing 19.2"
3 2 3 2 2 3 4 3 2 2 4 3 2 0			0.00	24"

I-JOIST HANGERS

- 1. Hangers shown illustrate the three to support 1-joists. most commonly used metal hangers
- 2. All nailing must meet the hanger manufacturer's recommendations.
- Hangers should be selected based on the joist depth, flange width and load capacity based on the maximum spans.
- Web stiffeners are required when the sides of the hangers do not laterally brace the top flange of the I-joist.







Face Mount

CCMC EVALUATION REPORT 13032-R

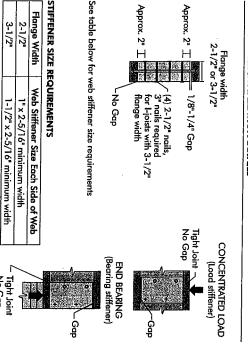
NORDIC I-JOIST SERIES

WEB STIFFENERS

RECOMMENDATIONS:

- A bearing stiffener is required in all the stiffener and the flange is at the top. engineered applications with factored -loist properties table tound of the I-joist reactions greater than shown in the Construction Guide (C101).The gap between
- support, the top flange. The gap between the stiffener and flange is at the top. the I-joist is supported in a hanger and the A bearing stiffener is required when sides of the hanger do not extend up to, and
- and the flange is at the bottom. A load stiffener is required at locations by the code. The gap between the stiffener adjusted for other load durations as permitted standard term load duration, and may be tip and the support. These values are tor cantilever, anywhere between the cantilever between supports, or in the case of a than 2,370 lbs is applied to the top flange where a tactored concentrated load greater
- SI units conversion: 1 inch = 25.4 mm

WEB STIFFENER INSTALLATION DETAILS



33 pieces per unit

33 pieces per unit

33 pieces per unit

23 pieces per unit

23 pieces per unit

23 pieces per unit

NPG Lumber 23 pieces per unit

S-P-F No.2 1950f MSR 2100f MSR 1950f MSR .Mc 8SO ↓ 2100f MSR 880 2400f MSR

finished product, reflects our commitment to quality. manufacturing process. Every phase of the operation, from total to the ... products to adhere to strict quality control procedures throughout the Chantiers Chibougamau Ltd. harvests its own trees, which enables. Northic

lumber in their flanges, ensuring consistent quality, superior streogy ்ழார். Nordic Engineered Wood I-joists use only finger-jointed black spruce longer span carrying capacity.

2019/04/16

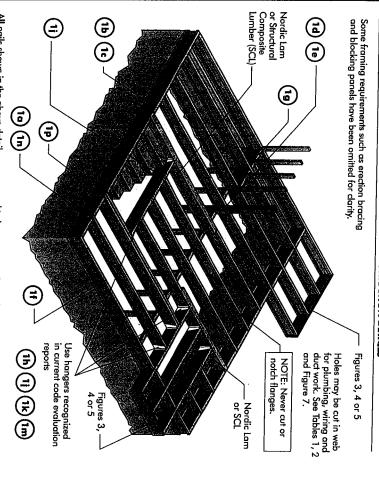
INSTALLING NORDIC I-JOISTS

- 1. Before laying out floor system components, verify that I-joist flange widths match hanger widths. If not, எழுத்தத்த
- 2. Except for cutting to length, I-joist flanges should never be cut, drilled, or notched.
- 3. Install Lioists so that top and bottom flanges are within 1/2 inch of true vertical alignment.
- 4. I-joists must be anchored securely to supports before floor sheathing is attached, and supports for multiple
- 5. Minimum bearing lengths: 1-3/4 inches for end bearings and 3-1/2 inches for intermediate bearings 150分子。
- 6. When using hangers, seat I-joists firmly in hanger bottoms to minimize settlement
- 7. Leave a 1/16-inch gap between the I-joist end and a header.
- 8. Concentrated loads greater than those that can normally be expected in residential construction should only be applied to the top surface of the top flange. Normal concentrated loads include track lighting fixtures, audio equipment and security cameras. Never suspend unusual or heavy loads from the I-joist's bottom flange. Whenever possible, suspend all concentrated loads from the top of the Lioist. Or, attach the load to blocking that has been securely fastened to the
- 9. Never install Lipists where they will be permanently exposed to weather, or where they will remain in direct contact with concrete or masonry.
- 10. Restrain ends of floor joists to prevent rollover. Use rim board, rim joists or l-joist blocking panels
- 11. For I-joists installed over and beneath bearing walls, use full depth blocking panels, rim board, or squash blocks (cripple members) to transfer gravity loads through the floor system to the wall or foundation below.
- 12. Due to shrinkage, common framing lumber set on edge may never be used as blocking or rim boards. Lipist blocking l-joist-compatible depth selected panels or other engineered wood products – such as rim board – must be cut to fit between the Lipists, and an
- 13. Provide permanent lateral support of the bottom flange of all Ljoists at interior supports of multiple-span joists. Similarly support the bottom flange of all cantilevered Lioists at the end support next to the cantilever extension. In the completed structure, the gypsum wallboard ceiling provides this lateral support. Until the final finished ceiling is applied, temporary bracing or struts must be used
- 14. If square-edge panels are used, edges must be supported between I-joists with 2x4 blocking. Glue panels to blocking to minimize squeaks. Blocking is not required under structural finish flooring, such as wood strip flooring, or if a separate underlayment layer is installed
- 15. Nail spacing: Space nails installed to the flange's top face in accordance with the applicable building code requirements or approved building plans.

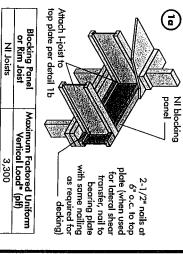
One 2-1/2"

FIGURE 1

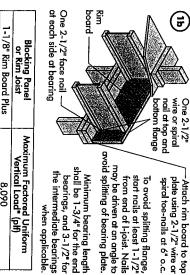
TYPICAL NORDIC I-JOIST FLOOR FRAMING AND CONSTRUCTION DETAILS



All nails shown in the above details are assumed to be common wire nails unless otherwise noted. 3" (0.122" dia.) common spiral nails may be substituted for 2-1/2" (0.128" dia.) common wire nails. Framing lumber assumed to be Spruce-Pine-Fir No. 2 or better. Individual components not shown to scale for clarity

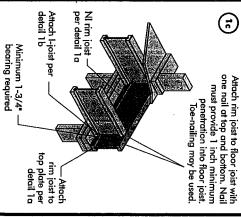


such as joist, header, or rafter. For concentrated vertical load transfer, see detail 1d
--



*The uniform vertical load is limited to a rim board depth of 16 inches or less and is based on standard term load duration. It shall not be			
is limited to a rim board depth of 16 inchestandard term load duration. It shall not be	or less and is based on s	*The uniform vertical load	
ited to a rim board depth of 16 inches	stando	d is lim	
a rim board depth of 16 inches m load duration. It shall not he	ard ten	rited to	Ī
board depth of 16 inches	ᆰ	αŗ.	
depth of 16 inches	dura	board	
of 16 inches	<u>ē</u> .	depth	
inche:	sha	of 16	
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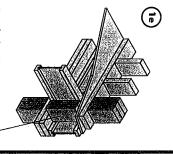
used in the design of a bending member, such as joist, header, or rafter. For concentrated vertical load transfer, see detail 1d.



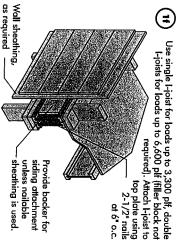
<u> </u>	3 5 3
Squash block -	ā
	Nt or rim board blocking panel per detail 1a —
	1/16" for squash blocks

6,600	4,300	1-1/8" Rim Board Plus
8,500	5,500	2x Lumber
5-1/2" wide	3-1/2" wide	
Maximum Factored Vertical per Pair of Squash Blocks (lbs)	Maximum Factored Vertical program of Squash Blocks (lbs)	Pair of Squash Blocks

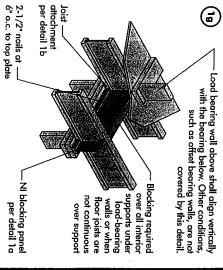
Provide lateral bracing per detail 1a, 1b, or 1c

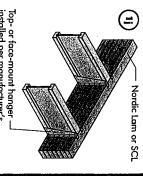


to post above. bearing below. Install squash blocks per detail 1d. Match bearing area of blocks below Transfer load from above to



required when rim board is used. Bracing per code shall be carried to the foundation Rim board may be used in lieu of I-joists. Backer is not





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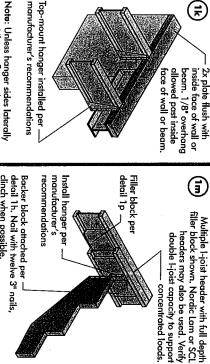
recommendations installed per manufacturer's

recommendations. beams, see the manufacturer's For nailing schedules for multiple

support the top flange, bearing stiffeners shall be used Note: Unless hanger sides laterally

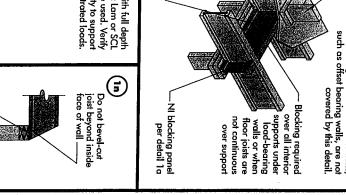
support the top flange, bearing

stiffeners shall be used



Maximum support capacity = 1,620 lbs. clinch when possible.

> tor clarity support, not shown at bearing for lateral

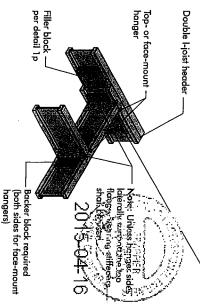


l-joist per detail 1b Attach

Note: Blocking required

 \bigcirc backer block will fit. Clinch. Install backer tight to top flange. Use twelve 3" nails, clinched when possible. Maximum factored Before installing a backer block to a double I-joist, drive three additional 3" nails through the webs and filler block where the Backer block (use if hanger load exceeds 360 lbs)

resistance for hanger for this detail = 1,620 lbs.



For hanger capacity see hanger manufacturer's recommendations. Verify double I-joist capacity to support concentrated loads.

nailing without splitting) BACKER BLOCKS (Blocks must be long enough to permit required

1-1/2"	Flange Width	Material Thickness Required*	Minimum Depth**
1-1/2"	2-1/2"	-	5-1/2"
	3-1/2"	1-1/2"	7-1/4"

- better for solid sawn lumber and wood structural panels conforming to CAN/CSA-O325 or CAN/CSA-O437 Standard. Minimum grade for backer block material shall be S-P-F No. 2 or
- ** For face-mount hangers use net joist depth minus 3-1/4" for joists with 1-1/2" thick flanges. For 2" thick flanges use net depth minus 4-1/4"

(Ŧ

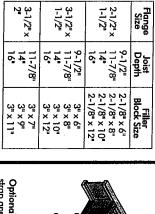
-One 2-1/2" nails at top and bottom flange

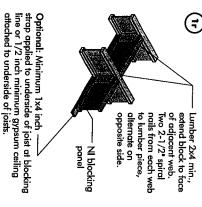
Two 2-1/2" nails from each web to

board

—2x4 min. (1/8" gap minimum)

lumber piece



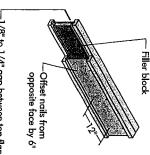


for spacing of the blocking.

All nails are common spiral in this detail the starter joist. Where required, see local code requirements the first joist space (or first and second joist space) next to In some local codes, blocking is prescriptively required in

One 2-1/2" nails one side only 2-1/2" nails at 6" o.c.

I-joist blocking panel



—1/8" to 1/4" gap between top flange and filler block

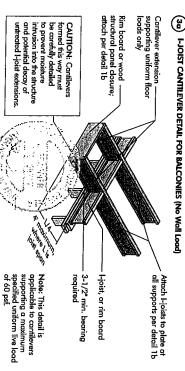
(

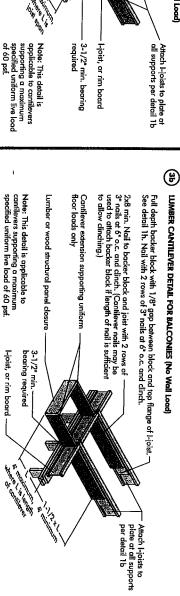
1. Support back of I-joist web during nailing to prevent damage to web/flange connection.

> DOUBLE I-JOIST CONSTRUCTION FILLER BLOCK REQUIREMENTS FOR

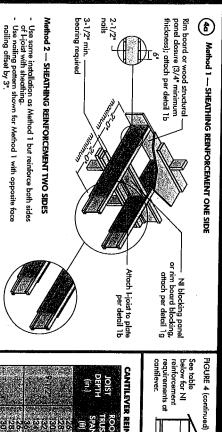
- Leave a 1/8 to 1/4-inch gap between top of filler block and bottom of top I-joist
- Filler block is required between joists for full length of span.
- Nail joists together with two rows of 3" are required. can be clinched, only two nails per foot Total of four nails per foot required. If nails possible) on each side of the double 1-joist nails at 12 inches o.c. (clinched when
- The maximum factored load that may be using this detail is 860 lbf/ft. Verify double applied to one side of the double joist

CANTILEVER DETAILS FOR BALCONIES (NO WALL LOAD)

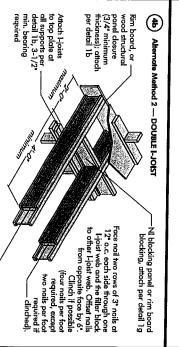




CANTILEVER DETAILS FOR VERTICAL BUILDING OFFSET (CONCENTRATED WALL LOAD)



Notes: Canadian softwood plywood sheathing or equivalent (minimum thickness 3/4") required on sides of joist. Depth shall match the full height of the joist. Nail with 2-1/2" nails at 6" o.c., top and bothom flange. Install with face grain horizontal. Attach I-joist to plate at all supports per detail 1b. Verify reinforced I-joist capacity.



Block Hoists together with filler blocks for the full length of the reinforceme For Hoist flange widths greater than 3 inches place an additional row of centreline of the reinforcing panel from each side. Clinch when possible.

CANTILEVER REINFORCEMENT METHODS ALLOWED

cantilever.

See table

Roof truss _ span

cantilever 21-01

> SSDIL Girder / Roof trusses

ب 13^LO" maximum - Jack trusses

Roof truss span

For hip roofs with the jack trusses running parallel to the cantilevered floor joists, the i-joist reinforcement requirements for a span of 26 ft. shall be permitted to be used.

maximum cantilever 2-0

44 P.75			1577 8 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	9 7 7 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8	JOIST RO DEPTH TRL (in.) (fi
*******					- 0 п
ZZZZZ			ZZZZZZZ	ZZ	30 psf, DL = 15 IST SPACING (in 16 19.2
		1.50	222222 222222	e e e e e e e e e e e e e e e e e e e	ROOI psf L) 24 12
ZZZZZ	2722				F LOADING (U L = 40 psf, DL JOIST SPACIN 16 1
7.7.7 1.5 16,0	2222				JNFACTORED) L = 15 psf L = 16 psf NG (in.) 19.2 24
22722 - 2222		7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7	222222 Q-2-122		LL = 50 p JOIST S
	222	2 - 2 - 2 - 2 - 2 - 2 - 2 - 2 - 2 - 2 -			sf, DL = 15 ps \$PACING (in.)
×เงเกเก=	Š	¥8005±≥	******	XXXX	if 24

- 1 = NI reinforced with 3/4* wood shuchural panel on one side only.
 2 = NI reinforced with 3/4* wood shuchural panel on both sides, or double I-joist.
 X = Try a deeper joist or closer spacing.
 Awarmum design load shall be: 15 per foor deed load, 55 per floor trott load, and 80 pff well load. Wall load is based on 3.0*

width window or door openings.

12" a.c. requirements for lesser spacing.

- ror larger openings, or multiple 3:0° width openings spaced less than 6:0° o.c., addi. it and joists beneath the opening's cripple studs may be required.

 3. Table applies to joists 12° to 24° o.c. that meet the floor span requirements for a design live load of 40 per and dead load of 15 psf. and a live load deflection limit of 1/480. Use
- 4. For conventional roof construction using a ridge beam, the Roof Truss Span column above is equivalent to the distance between the supporting wall and the ridge beam. When the roof is framed using a ridge board, the Roof Truss Span is equivalent to the distance between the supporting walls as if a distance between the supporting walls as if a
- truss is used.

 5. Cantilevered joists supporting girder trusses and hearns may require additional

RULES FOR CUTTING HOLES AND DUCT CHASE OPENINGS:

- The distance between the inside edge of the support and the centreline of any Table 1 or 2, respectively. hole or duct chase opening shall be in compliance with the requirements of
- ?2 l-joist top and bottom flanges must NEVER be cut, notched, or otherwise modified.
- ω Whenever possible, field-cut holes should be centred on the middle of the web.
- 4 The maximum size hole or the maximum depth of a duct chase opening that can between the top or bottom of the hole or opening and the adjacent I-joist flange. the l-joist minus 1/4 inch. A minimum of 1/8 inch should always be maintained be cut into an I-joist web shall equal the clear distance between the flanges of
- Ċ The sides of square holes or longest sides of rectangular holes should not exceed 3/4 of the diameter of the maximum round hole permitted at that location.
- 6. Where more than one hole is necessary, the distance between adjacent hole edges shall exceed twice the diameter of the largest round hole or twice the opening shall be sized and located in compliance with the requirements of longest rectangular hole or duct chase opening) and each hole and duct chase size of the largest square hole (or twice the length of the longest side of the Tables 1 and 2, respectively
- .7 A knockout is **not** considered a hole, may be utilized anywhere it occurs, and and/or duct chase openings. may be ignored for purposes of calculating minimum distances between holes
- œ Holes measuring 1-1/2 inches or smaller shall be permitted anywhere in a cantilevered section of a joist. Holes of greater size may be permitted subject to
- ? A 1-1/2 inch hole or smaller can be placed anywhere in the web provided that it meets the requirements of rule number 6 above.
- 11. Limit three maximum size holes per span, of which one may be a duct chase 10. All holes and duct chase openings shall be cut in a workman-like manner in accordance with the restrictions listed above and as illustrated in Figure 7.
- 12. A group of round holes at approximately the same location shall be permitted if they meet the requirements for a single round hole circumscribed around them.
- FIGURE 7

FIELD-CUT HOLE LOCATOR

bearing distance from for minimum See Table 1

2x diameter of larger hole

2x duct chase

Duct chase opening (see Table 2 for minimum distance

are :

whichever is length or hole

from bearing)

LOCATION OF CIRCULAR HOLES IN JOIST WEBS

Simple or Multiple Span for Dead Loads up to 15 psf and Live Loads up to 40 psf

	F			Joist Depti
		ot Hotele		
				G.
				∄ 5'
				¥ 5
A CONTRACTOR OF THE PERSON NAMED IN				m inside Round 5-1/4 7
STATE OF STATE		国际的关系是是企业	1111	ie of a ie dia 8
			11111	ny suppo meter (in. 8-5/8
				rt to cent) 9 10
		in großt.		re of hole
推动场间				(ft-in.)
2 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		(albia) karaj		12-3/4
				Span adjustmer Factor

- Notes whe may be used for I-plat spacing of 24 inches on centre or less.
 Hole location distance is measured from inside face of supports to centre of hole.
 Distances in this chart are based on uniformly loaded joists.

OPTIONAL:

The above table is based on the Ljoists used at their maximum span. If the Ljoists are placed at less than their full maximum span (see Maximum Scan Spans). The minimum distance from the centreline of the hole to the tace of any support (D) as given above may be reduced as follows:

Dreduced = Lactual × D

- Where:
- Dreduced Distance from the inside face of any support to centre of hole, reduced for less-than-maximum span applications (fit. The text is distance shall not be less than 6 inches from the face of the support to edge of the hole.
- The actual measured span distance between the inside faces of supports (ff)
- Ħ Span Adjustment Factor given in this table.
- <u>vactual</u> is greater than 1, use 1 in the above calculation for <u>badual</u>.

 SAF

<u>ത</u>

The minimum distance from the inside face of any support to centre of hole from this table

spaced 15 inches on centre along the length of the I-joist. Where possible, it is preferable to use knockouts instead of electrical or small plumbing lines. They for the contractor Knockouts are prescored holes provided /2 inches in diameter, and are r's convenience to install



should be cut with a notch the flange, or Holes in webs over-cut the web.

Never drill, cut or

sharp saw.

the holes is another good method to and then making the cuts between diameter hole in each of the four corners the rectangular hole by drilling a 1-inch stress concentrations. Slightly rounding the corners is recommended. Starting the corners, as this can cause unnecessary minimize damage to the I-joist For rectangular holes, avoid over-cutting

TABLE 2

DUCT CHASE OPENING SIZES AND LOCATIONS — Simple Span Only

		Minim	m distan	e from						
Joist Depth	Joist Series				Duct ch	ase leng	th (in.)	o centre c	n openin	g (n-in.)
Political and Service (See		8	10	12	14	16	18	20	22	24
		nu S	(1) (1)) () /	3.55		
	- 1 - 1		, ,						8.9	
				(+) (=)						
						0				
					j.		T.			
						100 PE 100	0.11	1000		
-							į			
	i			į						
		Y.								ik Fo
									10.7	
			el in	5						
								90	S	d
-								The state of the s	The second second	

- . Above table may be used for I-joist spacing of 24 inches on centre or less.

 Dud chase opening location distance is measured from inside face of supports to centre of opening.

 The above table is based on simple-span joists only. For other applications, contact your local distributor.

 Distances are based on uniformly loaded floor joists that meet the span requirements for a design live load of 40 psf and clead load of 15 psf, and a live load deflection limit of I/480. For other applications, contact your local distributor.

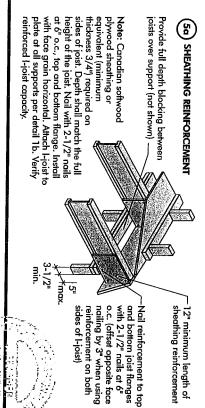
A knockout is **NOT** considered a hole, may be utilized wherever it occurs and may be ignored for purposes of calculating minimum distances

Knockouts

See rule 12

all duct chase openings and holes between top and bottom flange — Maintain minimum 1/8" space

BRICK CANTILEVER DETAILS FOR VERTICAL BUILDING OFFSET (CONCENTRATED WALL LOAD)



DEPIH (in.)

16

19.2

24

12

19.2

24

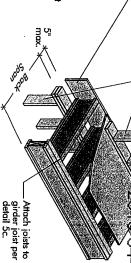
LL = 50 psf, DL = 15 psf JOIST SPACING (in.)

(5b) SET-BACK DETAIL

Bearing walls

attach per detail 1b. structural panel closure (3/4" minimum thickness), Rim board or wood

- Provide full depth blocking (not shown for clarity) between joists over support
- supports per detail 1b. Attach I-joist to plate at all
- 3-1/2" minimum I-joist bearing required

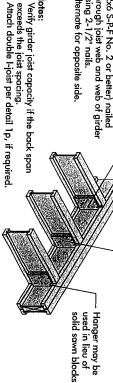


(F) SET-BACK CONNECTION

through joist web and web of girder using 2-1/2" nails. (2x6 S-P-F No. 2 or better) nailed Vertical solid sawn blocks

> bottom tlanges. naıls, toe-nail at top and Nail joist end using 3"





For larger openings openings spaced lea additional joists ber

N = No reinforcement required.
1 = NI reinforced with 3/4" wood structural

panel on one side only.

2 = NI reinforced with 3/4" wood structural

panel on both sides, or double I-joist.
X = Try a deeper joist or closer spacing.
2. Maximum design load shall be: 15 psf roof wall load. Wall load is based on 3'-0" naximum width window or door openings 55 pst floor total load, and 80 pff

BRICK CANTILEVER REINFORCEMENT METHODS ALLOWED Roof truss span LL = 30 psf, DL = 15 psf JOIST SPACING (in. 16 7 2'-0" Lmaximum -5" maximum cantilever 19.2 Truss Girder . 24 ROOF LOADING (UNFACTORED) 12 LL = 40 psf, DL = 15 psfJOIST SPACING (in.)

4. For conventional ridge beam, the above is equivalent the supporting to the supporting tof the supporting to the supporting to the supporting to the suppo

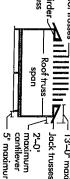
the floor span requirements for a design live load of 40 psf and dead load of 15 psf, and a live load deflection limit of L/480. Use 12" o.c. requirements for lesser spacing

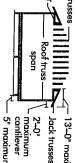
3. Table applies to joists 12" to 24" o.c. that meet the Roof Truss Span is equivalent to the the supporting wall and the ridge beam. When the roof is framed using a ridge board, nal roof construction using a ne Roof Truss Span column alent to the distance between

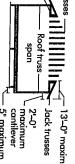
Cantilevered joists supporting girder trusses or truss is used. oof beams may require additional reinforcing.

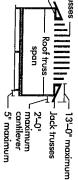
distance between the supporting walls as if a

FIGURE 5 (continued) below for NI See table cantilever. requirements at reinforcement Roof trusses









requirements for a span of 26 ft. shall be permitted to the I-joist reinforcement the cantilevered floor joists, trusses running parallel to For hip roofs with the jack

INSTALLING THE GLUED FLOOR SYSTEM

- 1. Wipe any mud, dirt, water, or ice from I-joist flanges before gluing
- 2. Snap a chalk line across the I-joists four feet in from the wall for panel edge alignment and as a boundary for spreading glue.
- Spread only enough glue to lay one or two panels at a time, or follow specific recommendations from
- 4. Lay the first panel with tongue side to the wall, and nail in place. This protects the tongue of the next panel from damage when tapped into place with a block and sledgehammer.
- 5. Apply a continuous line of glue (about 1/4-inch diameter) to the top flange of a single I-joist. Apply glue in a winding pattern on wide areas, such as with double 1-joists.
- 6. Apply two lines of glue on I-joists where panel ends but to assure proper gluing of each end.
- After the first row of panels is in place, spread glue in the groove of one or two panels at a time before laying the next row. Glue line may be continuous or spaced, but avoid squeeze-out by applying a thinner line (1/8 inch) than used on I-joist flanges.
- 8. Tap the second row of panels into place, using a block to protect groove edges.
- Stagger end joints in each succeeding row of panels. A 1/8-inch space between all end joints and nail to assure accurate and consistent spacing.) 1/8-inch at all edges, including T&G edges, is recommended. (Use a spacer tool or an 2-1/2" common
- 10. Complete all nailing of each panel before glue sets. Check the manufacturer's recommendations finished deck can be walked on right away and will carry construction loads without damage to the for cure time. (Warm weather accelerates glue setting.) Use 2" ring- or screw-shank nails for panels 3/4-inch thick or less, and 2-1/2" ring- or screw-shank nails for thicker panels. Space nails per the table below. Closer nail spacing may be required by some codes, or for diaphragm construction. The

FASTENERS FOR SHEATHING AND SUBFLOORING(1)

24 3/4	20 5/8	16 5/8	Maximum Minimum Joist Panel Spacing Thickness (in.) (in.)
2	2.	2"	Common Wire or Spiral Nails
1-3/4*	1-3/4"	1-3/4"	ail Size and Ty Ring Thread Nails or Screws
2"	2"	2"	/pe Staples
6	6	6"	Maximun of Fas Edges
12"	12"	12"	n Spacing teners Interm, Supports

- 1. Fasteners of sheathing and subflooring shall conform to the above table.
- 2. Staples shall not be less than 1/16-inch in diameter or thickness, with not less than a 3/8-inch crown driven with the crown parallel to framing.
- 3. Flooring screws shall not be less than 1/8-inch in diameter
- 4. Special conditions may impose heavy traffic and concentrated loads that require construction in excess of the minimums shown.
- 5. Use only adhesives conforming to CAN/CGSB-71.26 Standard, Adhesives for Field-Gluing Plywood to OSB panels with sealed surfaces and edges are to be used, use only solvent-based glues; check with Lumber Framing for Floor System, applied in accordance with the manufacturer's recommendations. If

Ref.: NRC-CNRC, National Building Code of Canada 2010, Table 9.23.3.5

IMPORTANT NOTE:

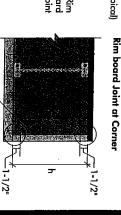
Floor sheathing must be field glued to the L-joist flanges in order to achieve the maximum spans shown in this document. If sheathing is nailed only, L-joist spans must be verified with your local distributor.

RIM BOARD INSTALLATION DETAILS

(8a) ATTACHMENT DETAILS WHERE RIM BOARDS ABUT

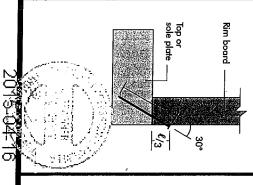
Rim board Joint Between Floor Joists (1) 2-1/2" nail 2-1/2" nails at 6" o.c. (typical)





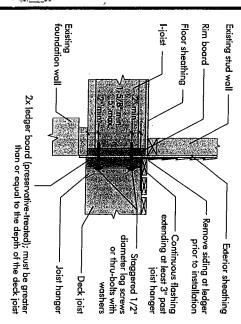
(B) TOE-NAIL CONNECTION AT RIM BOARD

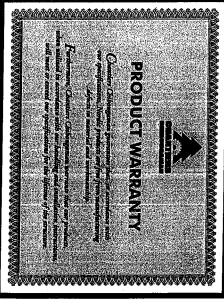
2-1/2" toe-nails at 6" o.c. (typical) —

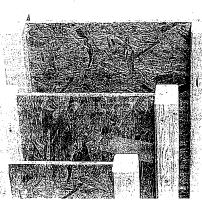


@ 2X LEDGER TO RIM BOARD ATTACHMENT DETAIL

Rim board joint -







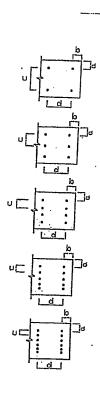
MICRO CITY

Engineering services inc.

TEL: (519) 287 - 2242

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, NOL 1MO

	LVL HEADER AND CONVENTIONAL					
		ER NAILING				
	DETAIL NUMBER	NUMBER OF ROWS	SPACING (INCHES o/c			
	Α	2.	12			
	В	2	8			
	С	2	6			
	D	2	4			
	1A	3	12			
	1B	3	8			
į	1C	3	. 6			
	1D	3	4 .			
	2A	4	. 12			
	2B	4	8			
۱	2C	4	6			
	2D	4	4			
	3A	5	12			
	3B	5	8			
	3C	5	. 6			
L	3D	5	4			
	4A	6 ,	12			
	4B	6	8			
L	4C	. 6	6			
L	4D	6	4			



NOTES:

- (1) MINIMUM LUMBER EDGE DISTANCE "a" = 1"
- (2) MINIMUM LUMBER END DISTANCE "b" = 2"
- (3) MINIMUM NAIL ROW SPACING "c" = 2"
- (4) STAGGER NAILS "d/2" BETWEEN PLIES FOR MULTI-PLY MEMBERS (3 PLY OR MORE)
- (5) ALL NAILS ARE 3-1/2" ARDOX SPIRAL NAILS
- (6) DO NOT USE AIR-DRIVEN NAILS



DNG NO TÄNNLOOI. 14

STRUCTURAL

COMPONENT DHLY

TO BE USED ONLY

WITH BEAM CALOS

BEARING THE

STAMP BELOWS

PROVICE NATLING
DETAIL Nº > SEE
OWO #TAMN1001-14