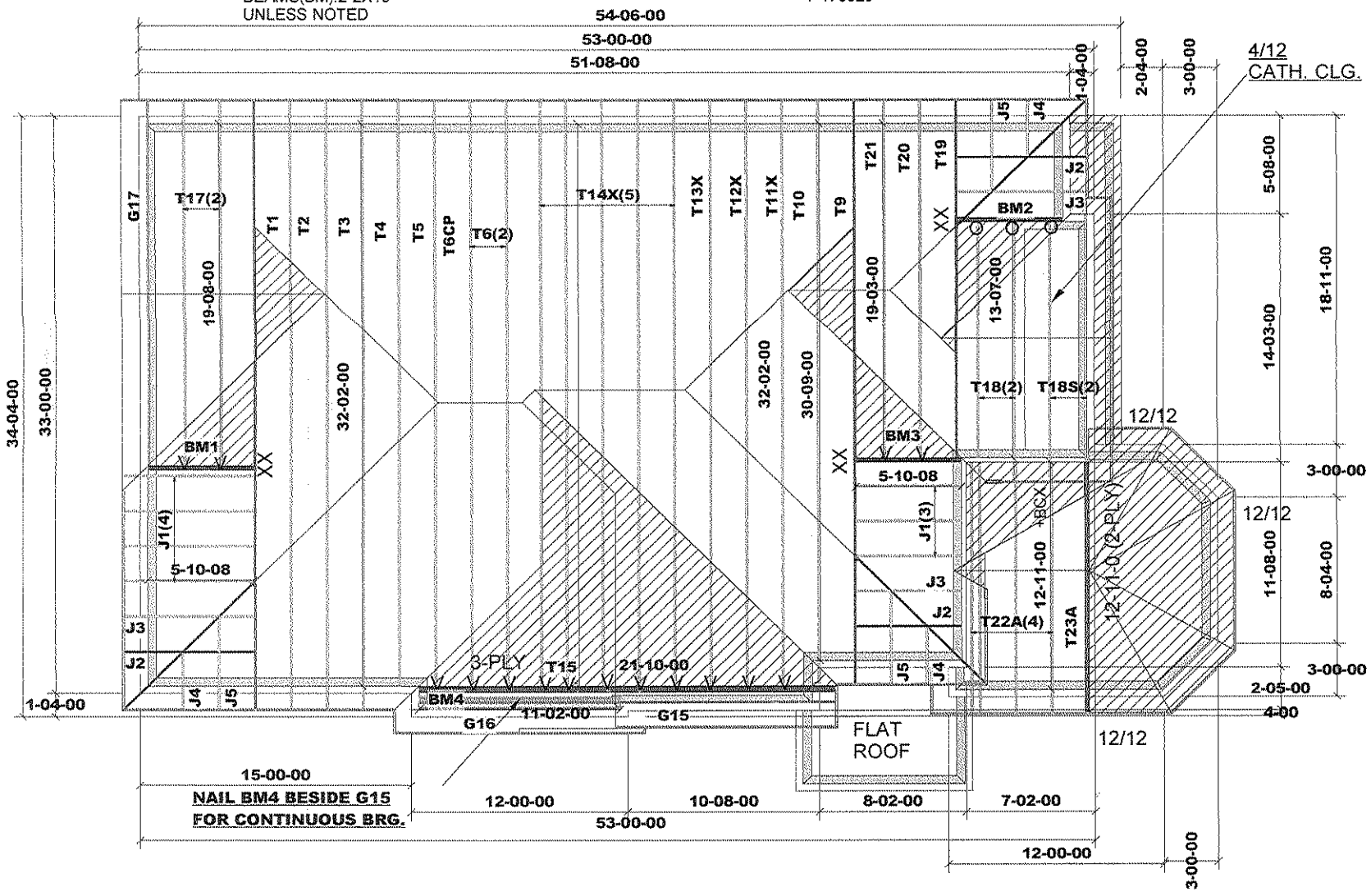
7/12 ROOF PITCH
BEAMS(BM):2-2X10
UNLESS NOTED

T-170329



12" FINISH O.H
R.T.M.C
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC. LATEST EDITION ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF @24"o.c.WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN CONFORMS WITH THE RELEVANT SECTION OF THE LATEST EDITION OF O.B.C. PART.9

DESIGN LOADS:
GROUND SNOW LOAD
Ss= 1.5 kPa
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

DENOTES CONVENTIONAL FRAMING

HARDWARE
LUS24(O)
LJS26DS(V)
HGUS26-2 (XX)

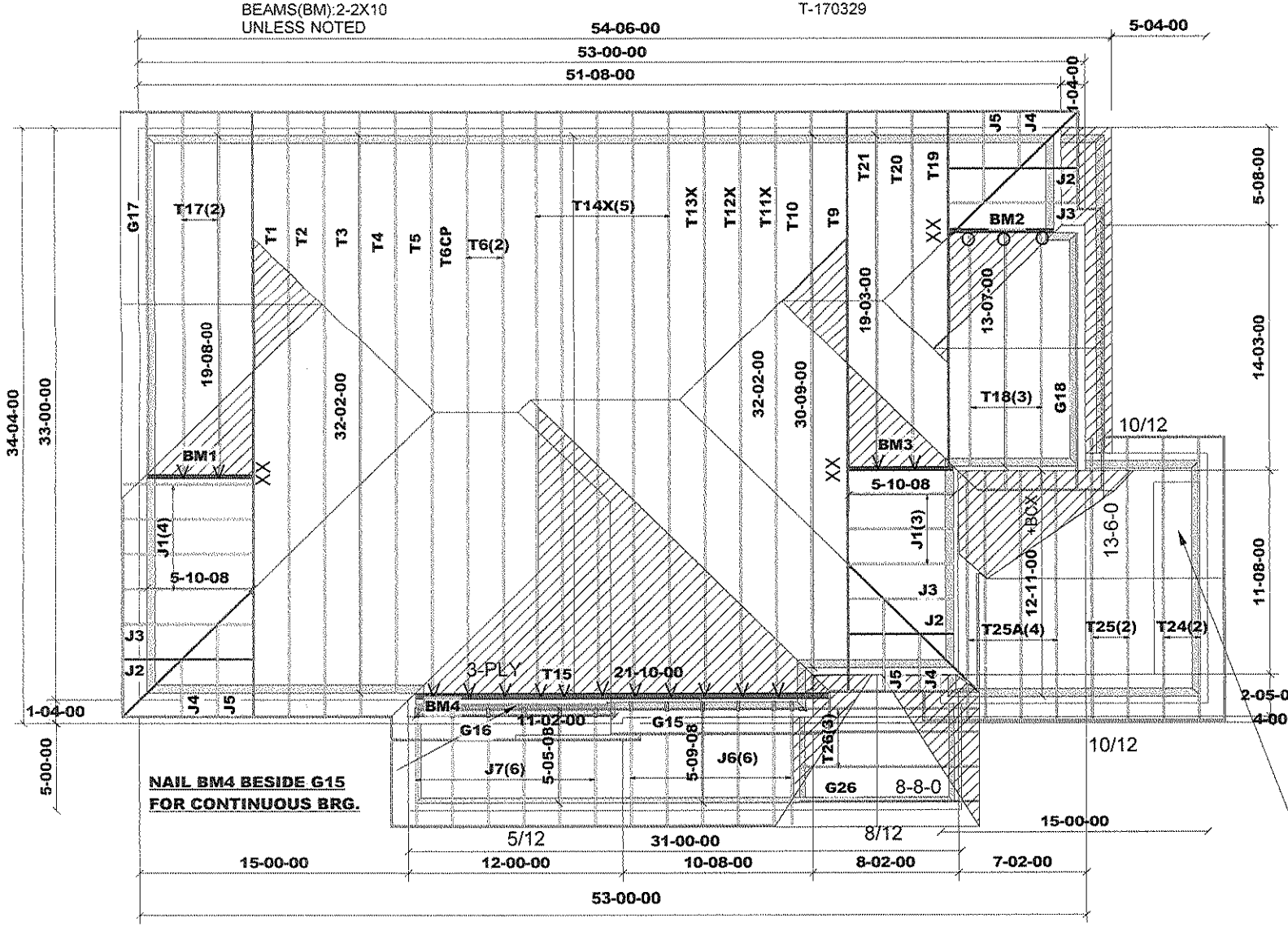
MID, 984

TAMARACK LUMBER INC.	Job Track: 43954	Builder / Location: GREENPARK HOMES / WATERDOWN
	Layout ID: 282176	Model / Elevation: ROSEWOOD 12 / 1
	Plan Log: 92173	Project: RUSSEL GARDENS PHASE 2
		Date: 8/30/2017 Designer: JG/S.V.E

Mitek ver 7.5.0

7/12 ROOF PITCH
BEAMS(BM):2-2X10
UNLESS NOTED

T-170329



12" FINISH O.H
R.T.M.C
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC. LATEST EDITION ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF @24"o.c.WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN CONFORMS WITH THE RELEVANT SECTION OF THE LATEST EDITION OF O.B.C. PART.9

DESIGN LOADS:
GROUND SNOW LOAD
Ss= 1.5 kPa
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

DENOTES CONVENTIONAL FRAMING

HARDWARE
LUS24(O)
LJS26DS(V)
HGUS26-2 (XX)

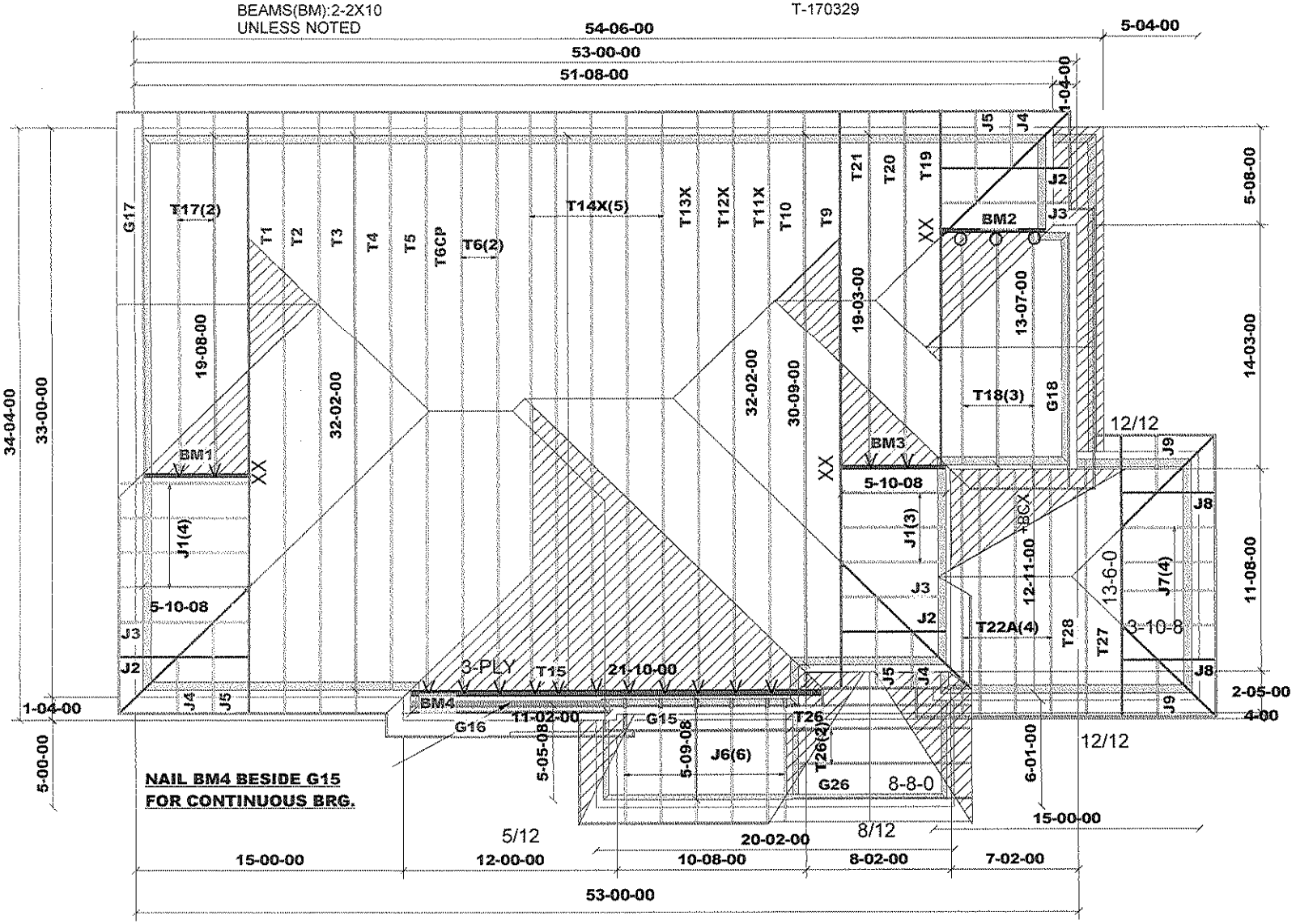
M10,984

TAMARACK LUMBER INC	Job Track: 43954	Builder / Location: GREENPARK HOMES / WATERDOWN
	Layout ID: 282177	Model / Elevation: ROSEWOOD 12 / 2
	Plan Log: 92173	Project: RUSSEL GARDENS PHASE 2
		Date: 8/30/2017 Designer: JG/S.V.E

Mitek ver 7.5.0

7/12 ROOF PITCH
BEAMS(BM):2-2X10
UNLESS NOTED

T-170329



12" FINISH O.H
R.T.M.C
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC. LATEST EDITION ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF @24"o.c.WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN CONFORMS WITH THE RELEVANT SECTION OF THE LATEST EDITION OF O.B.C. PART.9

DESIGN LOADS:
GROUND SNOW LOAD
Ss= 1.5 kPa

TC DEAD	3 PSF
BC LIVE	10.5 PSF
BC DEAD	7 PSF

DENOTES CONVENTIONAL FRAMING

HARDWARE
LUS24(O)
LJS26DS(V)
HGUS26-2 (XX)

M10,984

TAMARACK LUMBER INC.

Job Track: 43954
Layout ID: 282178
Plan Log: 92173

Builder / Location: GREENPARK HOMES / WATERDOWN
Project: RUSSEL GARDENS PHASE 2
Date: 8/30/2017 Designer: JG/S.V.E

Model / Elevation: ROSEWOOD 12 / 3

Mitek ver 7.5.0

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

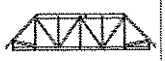
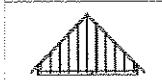
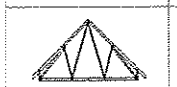
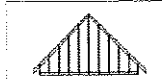
Delivery Shiplist

DATE	08/30/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 282176 LOCATION: WATERDOWN
BUILDER: GREENPARK - RUSSELL GARDENS PH 2 SUB-BUILDER:
MODEL: ROSEWOOD 12 ELEVATION: 1

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	1	T1 HIP GIRDER	7.00 0.00	32-02-00	04-08-09	2 X 4	2 X 6	01-03-08 01-03-08	01-03-07 01-03-07	178.21 111.00		
	1	T2 HIP	7.00 0.00	32-02-00	05-10-09	2 X 4	2 X 4	01-03-08 01-03-08	01-03-07 01-03-07	134.22 84.00		
	1	T3 HIP	7.00 0.00	32-02-00	07-00-09	2 X 4	2 X 4	01-03-08 01-03-08	01-03-07 01-03-07	137.24 85.83		
	1	T4 HIP	7.00 0.00	32-02-00	08-02-09	2 X 4	2 X 4	01-03-08 01-03-08	01-03-07 01-03-07	138.79 87.34		
	1	T5 HIP	7.00 0.00	32-02-00	09-04-09	2 X 4	2 X 4	01-03-08 01-03-08	01-03-07 01-03-07	139.68 86.33		
	2	T6 COMMON	7.00 0.00	32-02-00	10-08-00	2 X 4	2 X 4	01-03-08 01-03-08	01-03-07 01-03-07	283.50 176.66		
	1	T6CP COMMON	7.00 0.00	32-02-00	10-06-09	2 X 4	2 X 4	01-03-08 01-03-08	01-03-07 01-03-07	141.61 88.33		
	1	T9 HIP GIRDER	7.00 0.00	30-09-00	04-08-09	2 X 4	2 X 6	01-03-08 01-03-08	01-03-07 01-03-07	171.88 103.34		
	1	T10 HIP	7.00 0.00	30-09-00	05-10-09	2 X 4	2 X 4	01-03-08 01-03-08	01-03-07 01-03-07	123.10 76.00		
	1	T11X HIP	7.00 0.00	32-02-00	07-00-09	2 X 4	2 X 4	01-03-08 00-00-00	01-03-07 00-05-08	129.58 82.17		
	1	T12X HIP	7.00 0.00	32-02-00	08-02-09	2 X 4	2 X 4	01-03-08 00-00-00	01-03-07 00-05-08	130.61 81.17		
	1	T13X HIP	7.00 0.00	32-02-00	09-04-09	2 X 4	2 X 4	01-03-08 00-00-00	01-03-07 00-05-08	130.80 80.67		
	5	T14X COMMON	7.00 0.00	32-02-00	10-03-01	2 X 4	2 X 4	01-03-08 00-00-00	01-03-07 00-05-08	636.45 394.15		
	1 3 Ply	T15 COMMON	7.00 0.00	21-10-00	07-07-14	2 X 4	2 X 6	01-03-08 01-03-08	01-03-07 01-03-07	350.73 214.02		
	1	G15 GABLE	7.00 0.00	21-10-00	07-07-14	2 X 4	2 X 6	01-03-08 01-03-08	01-03-07 01-03-07	107.92 67.50		
	1	G16 COMMON	7.00 0.00	11-02-00	04-06-08	2 X 4	2 X 4	01-03-08 01-03-08	01-03-07 01-03-07	43.60 28.33		
	2	T17 COMMON	7.00 0.00	19-08-00	07-00-04	2 X 4	2 X 4	01-03-08 01-03-08	01-03-07 01-03-07	163.28 102.66		
	1	G17 GABLE	7.00 0.00	19-08-00	07-00-04	2 X 4	2 X 4	01-03-08 01-03-08	01-03-07 01-03-07	82.33 52.83		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

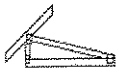
Delivery Shiplist

DATE	08/30/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 282176 LOCATION: WATERDOWN
BUILDER: GREENPARK - RUSSELL GARDENS PH 2 SUB-BUILDER:
MODEL: ROSEWOOD 12 ELEVATION: 1

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	2	T18 COMMON	7.00 / 0.00	13-07-00	05-03-00	2 X 4	2 X 4	01-03-08 / 01-03-08	01-03-07 / 01-03-07	112.88 / 73.00		
	2	T18S SCISSORS	7.00 / 4.00	13-07-00	05-03-00	2 X 4	2 X 4	01-03-08 / 01-03-08	01-03-07 / 01-03-07	111.36 / 74.34		
	1	T19 HIP GIRDER	7.00 / 0.00	19-03-00	04-08-09	2 X 4	2 X 6	01-03-08 / 01-03-08	01-03-07 / 01-03-07	89.11 / 55.33		
	1	T20 HIP	7.00 / 0.00	19-03-00	05-10-09	2 X 4	2 X 4	01-03-08 / 01-03-08	01-03-07 / 01-03-07	81.79 / 51.50		
	1	T21 COMMON	7.00 / 0.00	19-03-00	06-10-13	2 X 4	2 X 4	01-03-08 / 01-03-08	01-03-07 / 01-03-07	77.06 / 47.50		
	4	T22A COMMON	12.00 / 0.00	12-11-00	08-07-08	2 X 4	2 X 4	01-03-08 / 00-00-00	01-10-08 / 00-03-08	272.96 / 176.00		
	1 / 2 Ply	T23A COMMON	12.00 / 0.00	12-11-00	08-07-08	2 X 4	2 X 6	01-03-08 / 00-00-00	01-10-08 / 00-05-08	157.22 / 98.66		
	7	J1 JACK-OPEN	7.00 / 0.00	05-10-08	04-08-09	2 X 4	2 X 4	01-03-08 / 00-00-00	01-03-07 / 04-08-09	121.10 / 74.69		
	3	J2 JACK-OPEN	7.00 / 0.00	05-10-08	02-03-15	2 X 4	2 X 4	01-03-08 / -04-01-01	01-03-07 / 04-04-08	49.50 / 30.99		
	3	J3 JACK-OPEN	7.00 / 0.00	05-10-08	03-05-15	2 X 4	2 X 4	01-03-08 / -02-01-01	01-03-07 / 04-04-08	57.42 / 36.99		
	3	J4 JACK-OPEN	7.00 / 0.00	01-10-08	02-03-15	2 X 4	2 X 4	01-03-08 / -00-01-01	01-03-07 / 02-00-08	26.40 / 17.01		
	3	J5 JACK-OPEN	7.00 / 0.00	01-10-08	03-05-15	2 X 4	2 X 4	01-03-08 / 01-10-15	01-03-07 / 00-04-15	34.29 / 23.01		

TOTAL # TRUSS= **58.00** TOTAL BFT OF ALL TRUSSES= **2761.35** BFT. TOTAL WEIGHT OF ALL TRUSSES= **4414.62** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	HGUS26-2	
3	Hangers	LUS24	
15	Hangers	LJS26DS	

TOTAL # ITEMS= **21.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

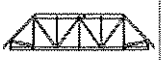
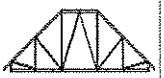
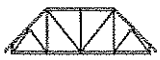
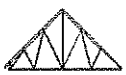
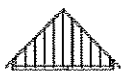
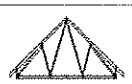
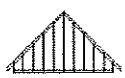
Delivery Shiplist

DATE	08/30/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 282177 LOCATION: WATERDOWN
BUILDER: GREENPARK - RUSSELL GARDENS PH 2 SUB-BUILDER:
MODEL: ROSEWOOD 12 ELEVATION: 2

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T1 HIP GIRDER	7.00 0.00	32-02-00	04-08-09	2 X 4	2 X 6	01-03-08 01-03-08	01-03-07 01-03-07	178.21 111.00		
	1	T2 HIP	7.00 0.00	32-02-00	05-10-09	2 X 4	2 X 4	01-03-08 01-03-08	01-03-07 01-03-07	134.22 84.00		
	1	T3 HIP	7.00 0.00	32-02-00	07-00-09	2 X 4	2 X 4	01-03-08 01-03-08	01-03-07 01-03-07	137.24 85.83		
	1	T4 HIP	7.00 0.00	32-02-00	08-02-09	2 X 4	2 X 4	01-03-08 01-03-08	01-03-07 01-03-07	138.79 87.34		
	1	T5 HIP	7.00 0.00	32-02-00	09-04-09	2 X 4	2 X 4	01-03-08 01-03-08	01-03-07 01-03-07	139.68 86.33		
	2	T6 COMMON	7.00 0.00	32-02-00	10-08-00	2 X 4	2 X 4	01-03-08 01-03-08	01-03-07 01-03-07	283.50 176.66		
	1	T6CP COMMON	7.00 0.00	32-02-00	10-06-09	2 X 4	2 X 4	01-03-08 01-03-08	01-03-07 01-03-07	141.61 88.33		
	1	T9 HIP GIRDER	7.00 0.00	30-09-00	04-08-09	2 X 4	2 X 6	01-03-08 01-03-08	01-03-07 01-03-07	171.88 103.34		
	1	T10 HIP	7.00 0.00	30-09-00	05-10-09	2 X 4	2 X 4	01-03-08 01-03-08	01-03-07 01-03-07	123.10 76.00		
	1	T11X HIP	7.00 0.00	32-02-00	07-00-09	2 X 4	2 X 4	01-03-08 00-00-00	01-03-07 00-05-08	129.58 82.17		
	1	T12X HIP	7.00 0.00	32-02-00	08-02-09	2 X 4	2 X 4	01-03-08 00-00-00	01-03-07 00-05-08	130.61 81.17		
	1	T13X HIP	7.00 0.00	32-02-00	09-04-09	2 X 4	2 X 4	01-03-08 00-00-00	01-03-07 00-05-08	130.80 80.67		
	5	T14X COMMON	7.00 0.00	32-02-00	10-03-01	2 X 4	2 X 4	01-03-08 00-00-00	01-03-07 00-05-08	636.45 394.15		
	1 3 Ply	T15 COMMON	7.00 0.00	21-10-00	07-07-14	2 X 4	2 X 6	01-03-08 01-03-08	01-03-07 01-03-07	350.73 214.02		
	1	G15 GABLE	7.00 0.00	21-10-00	07-07-14	2 X 4	2 X 6	01-03-08 01-03-08	01-03-07 01-03-07	107.92 67.50		
	1	G16 COMMON	7.00 0.00	11-02-00	04-06-08	2 X 4	2 X 4	01-03-08 01-03-08	01-03-07 01-03-07	43.60 28.33		
	2	T17 COMMON	7.00 0.00	19-08-00	07-00-04	2 X 4	2 X 4	01-03-08 01-03-08	01-03-07 01-03-07	163.28 102.66		
	1	G17 GABLE	7.00 0.00	19-08-00	07-00-04	2 X 4	2 X 4	01-03-08 01-03-08	01-03-07 01-03-07	82.33 52.83		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

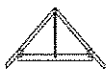
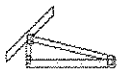
Delivery Shiplist

DATE	08/30/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 282177 LOCATION: WATERDOWN
BUILDER: GREENPARK - RUSSELL GARDENS PH 2 SUB-BUILDER:
MODEL: ROSEWOOD 12 ELEVATION: 2

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	3	T18 COMMON	7.00 0.00	13-07-00	05-03-00	2 X 4	2 X 4	01-03-08 01-03-08	01-03-07 01-03-07	169.32 109.50		
	1	G18 GABLE	7.00 0.00	13-07-00	05-03-00	2 X 4	2 X 4	01-03-08 01-03-08	01-03-07 01-03-07	54.34 35.50		
	1	T19 HIP GIRDER	7.00 0.00	19-03-00	04-08-09	2 X 4	2 X 6	01-03-08 01-03-08	01-03-07 01-03-07	89.11 55.33		
	1	T20 HIP	7.00 0.00	19-03-00	05-10-09	2 X 4	2 X 4	01-03-08 01-03-08	01-03-07 01-03-07	81.79 51.50		
	1	T21 COMMON	7.00 0.00	19-03-00	06-10-13	2 X 4	2 X 4	01-03-08 01-03-08	01-03-07 01-03-07	77.06 47.50		
	2	T24 SCISSOR	10.00 5.00	13-06-00	07-03-03	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	123.72 85.66		
	2	T25 COMMON	10.00 0.00	13-06-00	07-03-03	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	128.22 80.34		
	4	T25A COMMON	10.00 0.00	13-06-00	07-03-03	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 00-03-08	249.80 158.00		
	3	T26 COMMON	8.00 0.00	08-08-00	03-07-00	2 X 4	2 X 6	01-03-08 01-05-00	00-08-05 00-08-05	104.43 65.49		
	1	G26 GABLE	8.00 0.00	08-08-00	03-07-00	2 X 4	2 X 6	01-03-08 01-05-00	00-08-05 00-08-05	37.27 23.83		
	7	J1 JACK-OPEN	7.00 0.00	05-10-08	04-08-09	2 X 4	2 X 4	01-03-08 00-00-00	01-03-07 04-08-09	121.10 74.69		
	3	J2 JACK-OPEN	7.00 0.00	05-10-08	02-03-15	2 X 4	2 X 4	01-03-08 -04-01-01	01-03-07 04-04-08	49.50 30.99		
	3	J3 JACK-OPEN	7.00 0.00	05-10-08	03-05-15	2 X 4	2 X 4	01-03-08 -02-01-01	01-03-07 04-04-08	57.42 36.99		
	3	J4 JACK-OPEN	7.00 0.00	01-10-08	02-03-15	2 X 4	2 X 4	01-03-08 -00-01-01	01-03-07 02-00-08	26.40 17.01		
	3	J5 JACK-OPEN	7.00 0.00	01-10-08	03-05-15	2 X 4	2 X 4	01-03-08 01-10-15	01-03-07 00-04-15	34.29 23.01		
	6	J6 JACK-OPEN	5.00 0.00	05-09-08	02-09-00	2 X 4	2 X 4	01-03-08 00-00-00	00-04-01 00-06-01	93.00 55.98		
	6	J7 JACK-OPEN	5.00 0.00	05-05-08	02-07-06	2 X 4	2 X 4	01-03-08 00-00-00	00-04-01 00-06-01	88.20 55.98		

TOTAL # TRUSS= **76.00** TOTAL BFT OF ALL TRUSSES= **3009.63** BFT. TOTAL WEIGHT OF ALL TRUSSES= **4808.50** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	08/30/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 282177 LOCATION: WATERDOWN
BUILDER: GREENPARK - RUSSELL GARDENS PH 2 SUB-BUILDER:
MODEL: ROSEWOOD 12 ELEVATION: 2

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	HGUS26-2	
3	Hangers	LUS24	
15	Hangers	LJS26DS	

TOTAL # ITEMS= **21.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

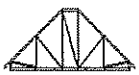
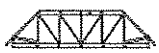
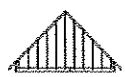
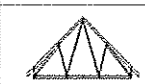
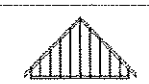
Delivery Shiplist

DATE	08/30/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 282178 LOCATION: WATERDOWN
BUILDER: GREENPARK - RUSSELL GARDENS PH 2 SUB-BUILDER:
MODEL: RESEWOOD 12 ELEVATION: 3

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	1	T1 HIP GIRDER	7.00 / 0.00	32-02-00	04-08-09	2 X 4	2 X 6	01-03-08 / 01-03-08	01-03-07 / 01-03-07	178.21 / 111.00		
	1	T2 HIP	7.00 / 0.00	32-02-00	05-10-09	2 X 4	2 X 4	01-03-08 / 01-03-08	01-03-07 / 01-03-07	134.22 / 84.00		
	1	T3 HIP	7.00 / 0.00	32-02-00	07-00-09	2 X 4	2 X 4	01-03-08 / 01-03-08	01-03-07 / 01-03-07	137.24 / 85.83		
	1	T4 HIP	7.00 / 0.00	32-02-00	08-02-09	2 X 4	2 X 4	01-03-08 / 01-03-08	01-03-07 / 01-03-07	138.79 / 87.34		
	1	T5 HIP	7.00 / 0.00	32-02-00	09-04-09	2 X 4	2 X 4	01-03-08 / 01-03-08	01-03-07 / 01-03-07	139.68 / 86.33		
	2	T6 COMMON	7.00 / 0.00	32-02-00	10-08-00	2 X 4	2 X 4	01-03-08 / 01-03-08	01-03-07 / 01-03-07	283.50 / 176.66		
	1	T6CP COMMON	7.00 / 0.00	32-02-00	10-06-09	2 X 4	2 X 4	01-03-08 / 01-03-08	01-03-07 / 01-03-07	141.61 / 88.33		
	1	T9 HIP GIRDER	7.00 / 0.00	30-09-00	04-08-09	2 X 4	2 X 6	01-03-08 / 01-03-08	01-03-07 / 01-03-07	171.88 / 103.34		
	1	T10 HIP	7.00 / 0.00	30-09-00	05-10-09	2 X 4	2 X 4	01-03-08 / 01-03-08	01-03-07 / 01-03-07	123.10 / 76.00		
	1	T11X HIP	7.00 / 0.00	32-02-00	07-00-09	2 X 4	2 X 4	01-03-08 / 00-00-00	01-03-07 / 00-05-08	129.58 / 82.17		
	1	T12X HIP	7.00 / 0.00	32-02-00	08-02-09	2 X 4	2 X 4	01-03-08 / 00-00-00	01-03-07 / 00-05-08	130.61 / 81.17		
	1	T13X HIP	7.00 / 0.00	32-02-00	09-04-09	2 X 4	2 X 4	01-03-08 / 00-00-00	01-03-07 / 00-05-08	130.80 / 80.67		
	5	T14X COMMON	7.00 / 0.00	32-02-00	10-03-01	2 X 4	2 X 4	01-03-08 / 00-00-00	01-03-07 / 00-05-08	636.45 / 394.15		
	1 / 3 Ply	T15 COMMON	7.00 / 0.00	21-10-00	07-07-14	2 X 4	2 X 6	01-03-08 / 01-03-08	01-03-07 / 01-03-07	350.73 / 214.02		
	1	G15 GABLE	7.00 / 0.00	21-10-00	07-07-14	2 X 4	2 X 6	01-03-08 / 01-03-08	01-03-07 / 01-03-07	107.92 / 67.50		
	1	G16 COMMON	7.00 / 0.00	11-02-00	04-06-08	2 X 4	2 X 4	01-03-08 / 01-03-08	01-03-07 / 01-03-07	43.60 / 28.33		
	2	T17 COMMON	7.00 / 0.00	19-08-00	07-00-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-03-07 / 01-03-07	163.28 / 102.66		
	1	G17 GABLE	7.00 / 0.00	19-08-00	07-00-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-03-07 / 01-03-07	82.33 / 52.83		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

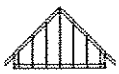
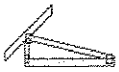
Delivery Shiplist

DATE	08/30/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 282178 LOCATION: WATERDOWN
BUILDER: GREENPARK - RUSSELL GARDENS PH2 SUB-BUILDER:
MODEL: RESEWOOD 12 ELEVATION: 3

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	3	T18 COMMON	7.00 0.00	13-07-00	05-03-00	2 X 4	2 X 4	01-03-08 01-03-08	01-03-07 01-03-07	169.32 109.50		
	1	G18 GABLE	7.00 0.00	13-07-00	05-03-00	2 X 4	2 X 4	01-03-08 01-03-08	01-03-07 01-03-07	54.34 35.50		
	1	T19 HIP GIRDER	7.00 0.00	19-03-00	04-08-09	2 X 4	2 X 6	01-03-08 01-03-08	01-03-07 01-03-07	89.11 55.33		
	1	T20 HIP	7.00 0.00	19-03-00	05-10-09	2 X 4	2 X 4	01-03-08 01-03-08	01-03-07 01-03-07	81.79 51.50		
	1	T21 COMMON	7.00 0.00	19-03-00	06-10-13	2 X 4	2 X 4	01-03-08 01-03-08	01-03-07 01-03-07	77.06 47.50		
	1	T27 HIP GIRDER	12.00 0.00	13-06-00	05-09-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	66.81 42.67		
	1	T28 HIP	12.00 0.00	13-06-00	07-09-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	71.52 46.00		
	4	T22A COMMON	12.00 0.00	12-11-00	08-07-08	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 00-03-08	272.96 176.00		
	3	T26 COMMON	8.00 0.00	08-08-00	03-07-00	2 X 4	2 X 6	01-03-08 01-05-00	00-08-05 00-08-05	104.43 65.49		
	1	G26 GABLE	8.00 0.00	08-08-00	03-07-00	2 X 4	2 X 6	01-03-08 01-05-00	00-08-05 00-08-05	37.27 23.83		
	7	J1 JACK-OPEN	7.00 0.00	05-10-08	04-08-09	2 X 4	2 X 4	01-03-08 00-00-00	01-03-07 04-08-09	121.10 74.69		
	3	J2 JACK-OPEN	7.00 0.00	05-10-08	02-03-15	2 X 4	2 X 4	01-03-08 -04-01-01	01-03-07 00-03-08	49.50 30.99		
	3	J3 JACK-OPEN	7.00 0.00	05-10-08	03-05-15	2 X 4	2 X 4	01-03-08 -02-01-01	01-03-07 00-03-08	57.42 36.99		
	3	J4 JACK-OPEN	7.00 0.00	01-10-08	02-03-15	2 X 4	2 X 4	01-03-08 -00-01-01	01-03-07 00-03-08	26.40 17.01		
	3	J5 JACK-OPEN	7.00 0.00	01-10-08	03-05-15	2 X 4	2 X 4	01-03-08 01-10-15	01-03-07 00-04-15	34.29 23.01		
	6	J6 JACK-OPEN	5.00 0.00	05-09-08	02-09-00	2 X 4	2 X 4	01-03-08 00-00-00	00-04-01 00-06-01	93.00 55.98		
	4	J7 JACK-OPEN	12.00 0.00	03-10-08	05-09-00	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 05-09-00	59.00 37.32		
	2	J8 JACK-OPEN	12.00 0.00	03-10-08	03-07-15	2 X 4	2 X 4	01-03-08 -02-01-01	01-10-08 00-03-08	29.08 18.66		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	08/30/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 282178 LOCATION: WATERDOWN
BUILDER: GREENPARK - RUSSELL GARDENS PH 2 SUB-BUILDER:
MODEL: RESEWOOD 12 ELEVATION: 3

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	2	J9 JACK-OPEN	12.00 0.00	01-10-08	03-07-15	2 X 4	2 X 4	01-03-08 -00-01-01	01-10-08 00-03-08	21.72 15.00		

TOTAL # TRUSS= **76.00** TOTAL BFT OF ALL TRUSSES= **2965.30** BFT. TOTAL WEIGHT OF ALL TRUSSES= **4739.65** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	HGUS26-2	
3	Hangers	LUS24	
15	Hangers	LSSU28	

TOTAL # ITEMS= **21.00**

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
282176	T1	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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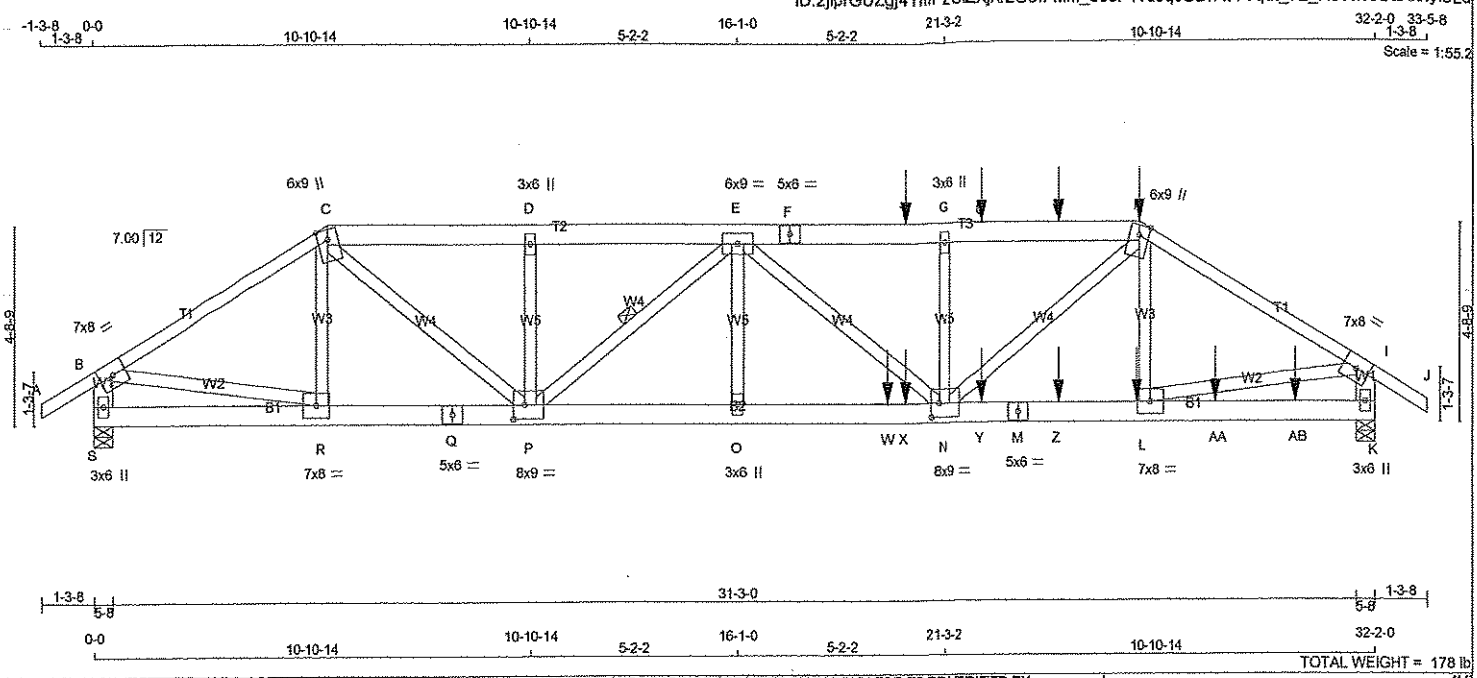

TOTAL WEIGHT = 178 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	1650F 1.5E	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x4	DRY	1650F 1.5E	SPF
S - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
S - Q	2x6	DRY	2100F 1.8E	SPF
Q - M	2x6	DRY	2100F 1.8E	SPF
M - K	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	7.0	8.0		
C	TTWW+m	MT20	6.0	9.0	Edge	
D	TMW+w	MT20	3.0	6.0		
E	TMWWW-t	MT20	6.0	9.0		
F	TS-t	MT20	5.0	6.0		
G	TMW+w	MT20	3.0	6.0		
H	TTWW+m	MT20	6.0	9.0	Edge	
I	TMVW-t	MT20	7.0	8.0		
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	7.0	8.0		
M	BS-t	MT20	5.0	6.0		
N	BMWWW-t	MT20	8.0	9.0	4.00	2.25
O	BMW+w	MT20	3.0	6.0		
P	BMWWW-t	MT20	8.0	9.0	4.00	3.25
Q	BS-t	MT20	5.0	6.0		
R	BMWW-t	MT20	7.0	8.0		
S	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 101.4 lbs FACTORED DOWN AT 20-2-12, 101.4 lbs FACTORED DOWN AT 22-2-12, AND 101.4 lbs FACTORED DOWN AT 24-2-12, AND 473.2 lbs FACTORED DOWN AT 26-3-8 ON TOP CHORD, AND 1423.5 lbs FACTORED DOWN AT 19-9-8, 70.9 lbs FACTORED DOWN AT 20-2-12, 70.9 lbs FACTORED DOWN AT 22-2-12, 70.9 lbs FACTORED DOWN AT 24-2-12, 70.9 lbs FACTORED DOWN AT 26-2-12, AND 86.2 lbs FACTORED DOWN AT 28-2-12, AND 86.2 lbs FACTORED DOWN AT 30-2-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	2702	0	2702	0	0	5-8	5-8
K	3600	0	3600	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	2190	1262 / 0	468 / 0	0 / 0	0 / 0	461 / 0	0 / 0
K	2928	1669 / 0	637 / 0	0 / 0	0 / 0	623 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.96 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 29	-84.3	-84.3	0.10 (1)	10.00
B- C	-3576 / 0	-84.3	-84.3	0.74 (1)	3.56
C- D	-5076 / 0	-84.3	-84.3	0.32 (1)	3.63
D- E	-5075 / 0	-84.3	-84.3	0.29 (1)	3.66
E- F	-6614 / 0	-84.3	-84.3	0.46 (1)	3.08
F- T	-6614 / 0	-84.3	-84.3	0.46 (1)	3.08
T- G	-6614 / 0	-84.3	-84.3	0.46 (1)	3.08
G- U	-6615 / 0	-84.3	-84.3	0.56 (1)	2.96
U- V	-6615 / 0	-84.3	-84.3	0.56 (1)	2.96
V- H	-6615 / 0	-84.3	-84.3	0.56 (1)	2.96
H- I	-4885 / 0	-84.3	-84.3	0.94 (1)	2.98
I- J	0 / 29	-84.3	-84.3	0.10 (1)	10.00
S- B	-2632 / 0	0.0	0.0	0.19 (1)	6.37
K- I	-3464 / 0	0.0	0.0	0.25 (1)	5.68
S- R	0 / 0	-28.0	-28.0	0.06 (2)	10.00
R- Q	0 / 3105	-28.0	-28.0	0.22 (1)	10.00
Q- P	0 / 3105	-28.0	-28.0	0.22 (1)	10.00
P- O	0 / 6481	-28.0	-28.0	0.48 (1)	10.00
O- W	0 / 6481	-28.0	-28.0	0.79 (1)	10.00
W- X	0 / 6481	-28.0	-28.0	0.79 (1)	10.00
X- N	0 / 6481	-28.0	-28.0	0.79 (1)	10.00
N- Y	0 / 4246	-28.0	-28.0	0.44 (1)	10.00
Y- M	0 / 4246	-28.0	-28.0	0.44 (1)	10.00
M- Z	0 / 4246	-28.0	-28.0	0.44 (1)	10.00
Z- L	0 / 4246	-28.0	-28.0	0.44 (1)	10.00
L-AA	0 / 0	-28.0	-28.0	0.12 (2)	10.00
AA-AB	0 / 0	-28.0	-28.0	0.12 (2)	10.00
AB- K	0 / 0	-28.0	-28.0	0.12 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R- C	-286 / 74	0.06 (1)
L- H	-502 / 158	0.11 (1)
B- R	0 / 3133	0.55 (1)
L- I	0 / 4280	0.76 (1)
N- H	0 / 3111	0.55 (1)
C- P	0 / 2588	0.46 (1)
N- G	-731 / 0	0.16 (1)
P- D	-496 / 0	0.11 (1)
E- N	0 / 172	0.03 (1)
P- E	-1820 / 0	0.40 (1)
O- E	0 / 609	0.11 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
H	26-3-8	-473	-473	---	FRONT	VERT	TOTAL
L	26-2-12	-41	-71	---	FRONT	VERT	TOTAL
T	20-2-12	-101	-101	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.07")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.31")
ALLOWABLE DEFL.(TL)= L/360 (1.07")
CALCULATED VERT. DEFL.(TL) = L/ 792 (0.49")

CSI: TC=0.94 (H-I:1) , BC=0.79 (N-O:1) , WB=0.76 (I-L:1) , SSI=0.77 (N-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (B) (INPUT = 0.90)
JSI METAL= 0.91 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER 83077 S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM 43989 17
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
282176	T1	1	1	TRUSS DESC.	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
U	22-2-12	-101	-101	---	FRONT	VERT	TOTAL
V	24-2-12	-101	-101	---	FRONT	VERT	TOTAL
W	19-9-8	-1424	-1424	---	FRONT	VERT	TOTAL
X	20-2-12	-41	-71	---	FRONT	VERT	TOTAL
Y	22-2-12	-41	-71	---	FRONT	VERT	TOTAL
Z	24-2-12	-41	-71	---	FRONT	VERT	TOTAL
AA	28-2-12	-49	-86	---	FRONT	VERT	TOTAL
AB	30-2-12	-49	-86	---	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 43989-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
282176	T2	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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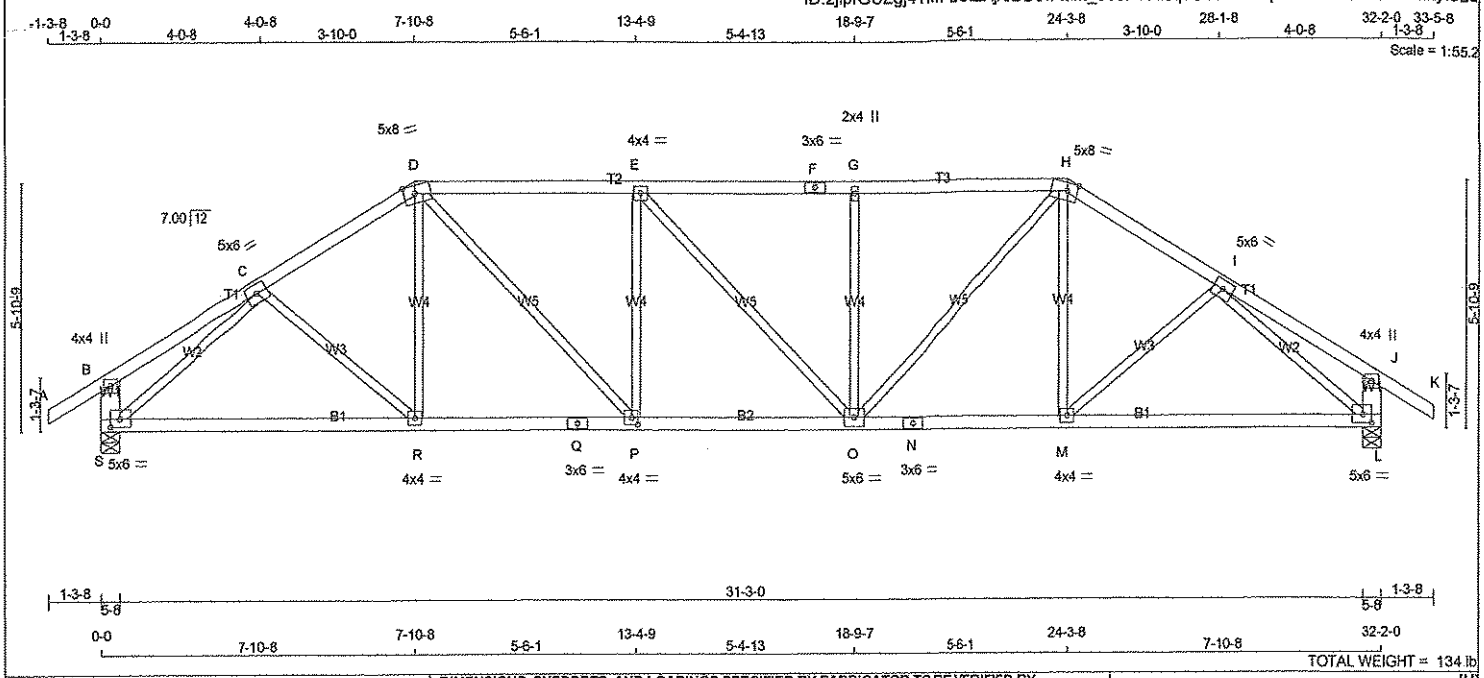

TOTAL WEIGHT = 134 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW-m	MT20	5.0	8.0	Edge	3.25
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	8.0	Edge	3.25
I	TMWW-t	MT20	5.0	6.0		
J	TMV+p	MT20	4.0	4.0		
L	BMVW1-t	MT20	5.0	6.0	2.25	2.75
M	BMWW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	6.0		
O	BMWWW-t	MT20	5.0	6.0		
P	BMWW-t	MT20	4.0	4.0	2.00	1.75
Q	BS-t	MT20	3.0	6.0		
R	BMWW-t	MT20	4.0	4.0		
S	BMVW1-t	MT20	5.0	6.0	2.25	2.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
S	1920	0	1920	0	0	5-8	5-8
L	1920	0	1920	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1560	893 / 0	338 / 0	0 / 0	0 / 0	330 / 0	0 / 0
L	1560	893 / 0	338 / 0	0 / 0	0 / 0	330 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.85 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 29	-84.3	-84.3	0.11 (1)	10.00	C- R	0 / 171	0.04 (3)
B- C	0 / 17	-84.3	-84.3	0.18 (1)	10.00	R- D	0 / 299	0.07 (2)
C- D	-2279 / 0	-84.3	-84.3	0.27 (1)	4.30	D- P	0 / 883	0.20 (1)
D- E	-2575 / 0	-84.3	-84.3	0.49 (1)	3.85	P- E	-497 / 0	0.27 (1)
E- F	-2574 / 0	-84.3	-84.3	0.48 (1)	3.85	E- O	-2 / 0	0.00 (1)
F- G	-2574 / 0	-84.3	-84.3	0.48 (1)	3.85	O- G	-497 / 0	0.26 (1)
G- H	-2574 / 0	-84.3	-84.3	0.48 (1)	3.86	O- H	0 / 881	0.20 (1)
H- I	-2279 / 0	-84.3	-84.3	0.27 (1)	4.30	M- H	0 / 300	0.07 (2)
I- J	0 / 17	-84.3	-84.3	0.18 (1)	10.00	M- I	0 / 171	0.04 (3)
J- K	0 / 29	-84.3	-84.3	0.11 (1)	10.00	S- C	-2475 / 0	0.99 (1)
S- B	-247 / 0	0.0	0.0	0.02 (1)	7.81	I- L	-2475 / 0	0.99 (1)
L- J	-247 / 0	0.0	0.0	0.02 (1)	7.81			
S- R	0 / 1904	-28.0	-28.0	0.59 (2)	10.00			
R- Q	0 / 1955	-28.0	-28.0	0.61 (2)	10.00			
Q- P	0 / 1955	-28.0	-28.0	0.61 (2)	10.00			
P- O	0 / 2575	-28.0	-28.0	0.50 (1)	10.00			
O- N	0 / 1955	-28.0	-28.0	0.61 (2)	10.00			
N- M	0 / 1955	-28.0	-28.0	0.61 (2)	10.00			
M- L	0 / 1904	-28.0	-28.0	0.59 (2)	10.00			

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM43990-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.07")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.19")
ALLOWABLE DEFL.(TL)= L/360 (1.07")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.31")

CSI: TC=0.49 (D-E:1) , BC=0.61 (M-O:2) , WB=0.99 (I-L:1) , SSI=0.22 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.62 (I) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43984	DRWG NO.
282176	T3	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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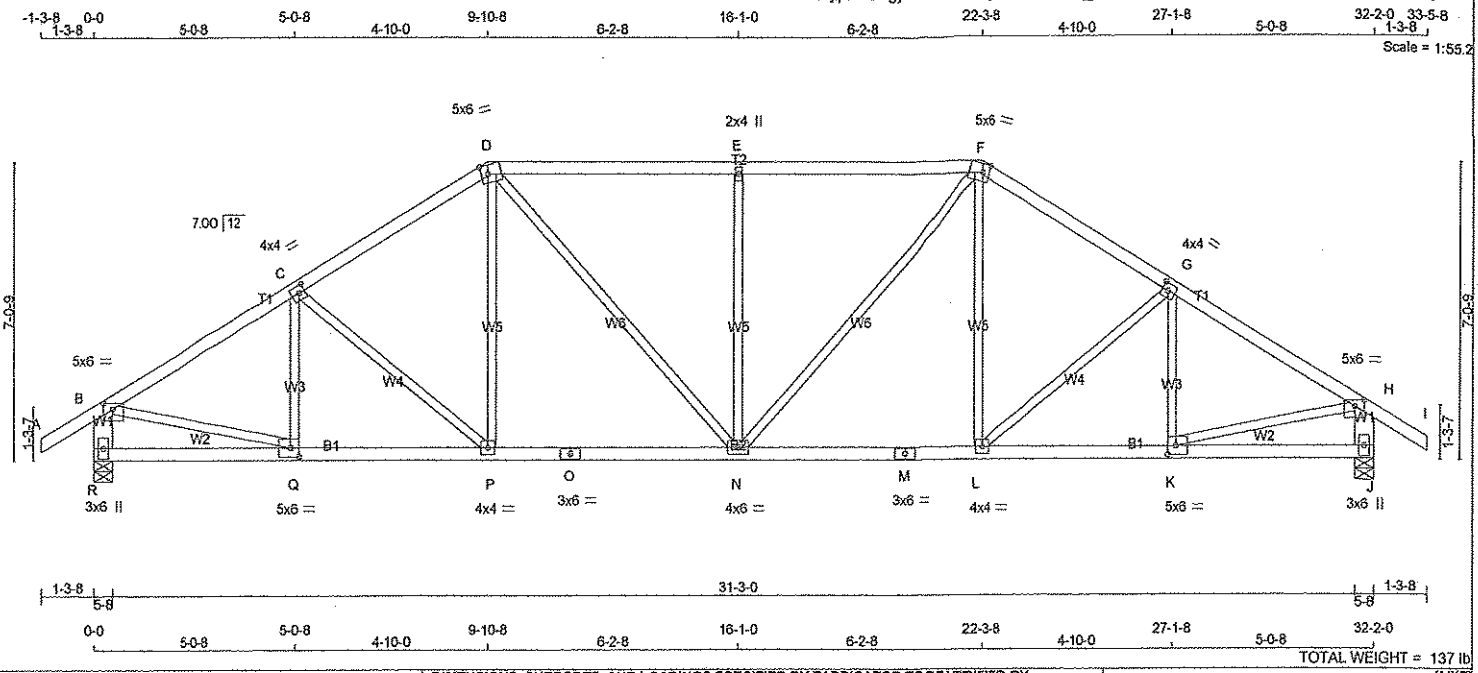

TOTAL WEIGHT = 137 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.75	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	6.0	2.50	2.00
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.50	2.00
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
H	TMVW-p	MT20	5.0	6.0	1.75	2.75
J	BMV1+p	MT20	3.0	6.0		
K	BMWW-t	MT20	5.0	6.0	2.50	2.50
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	6.0		
O	BS-t	MT20	3.0	6.0		
P	BMWW-t	MT20	4.0	4.0		
Q	BMWW-t	MT20	5.0	6.0	2.50	2.50
R	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
R	1920	0	1920	0	0	5-8	5-8
J	1920	0	1920	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1560	893 / 0	338 / 0	0 / 0	0 / 0	330 / 0	0 / 0
J	1560	893 / 0	338 / 0	0 / 0	0 / 0	330 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.07 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
FR-TO		FROM TO			FR-TO
A- B	0 / 29	-84.3 -84.3	0.11 (1)	10.00	
B- C	-2331 / 0	-84.3 -84.3	0.34 (1)	4.20	
C- D	-2156 / 0	-84.3 -84.3	0.32 (1)	4.35	
D- E	-2219 / 0	-84.3 -84.3	0.50 (1)	4.07	
E- F	-2219 / 0	-84.3 -84.3	0.50 (1)	4.07	
F- G	-2156 / 0	-84.3 -84.3	0.32 (1)	4.35	
G- H	-2331 / 0	-84.3 -84.3	0.34 (1)	4.20	
H- I	0 / 29	-84.3 -84.3	0.11 (1)	10.00	
R- B	-1859 / 0	0.0 0.0	0.12 (1)	7.34	
J- H	-1859 / 0	0.0 0.0	0.12 (1)	7.34	
R- Q	0 / 0	-28.0 -28.0	0.15 (3)	10.00	
Q- P	0 / 2034	-28.0 -28.0	0.42 (1)	10.00	
P- O	0 / 1844	-28.0 -28.0	0.44 (2)	10.00	
O- N	0 / 1844	-28.0 -28.0	0.44 (2)	10.00	
N- M	0 / 1844	-28.0 -28.0	0.44 (2)	10.00	
M- L	0 / 1844	-28.0 -28.0	0.44 (2)	10.00	
L- K	0 / 2034	-28.0 -28.0	0.42 (1)	10.00	
K- J	0 / 0	-28.0 -28.0	0.15 (3)	10.00	

WEBS

MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-248 / 68	0.07 (1)
C- P	-248 / 0	0.18 (1)
P- D	0 / 389	0.08 (2)
D- N	0 / 554	0.12 (1)
N- E	-641 / 0	0.55 (1)
N- F	0 / 554	0.12 (1)
L- F	0 / 389	0.08 (2)
L- G	-248 / 0	0.18 (1)
K- G	-248 / 68	0.07 (1)
B- Q	0 / 2071	0.47 (1)
K- H	0 / 2071	0.47 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.07")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (1.07")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.50 (E-F:1) , BC=0.44 (N-P:2) , WB=0.55 (E-N:1) , SSI=0.25 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (F) (INPUT = 0.90)
JSI METAL= 0.54 (O) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER 83077 S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM4399/-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
282176	T4	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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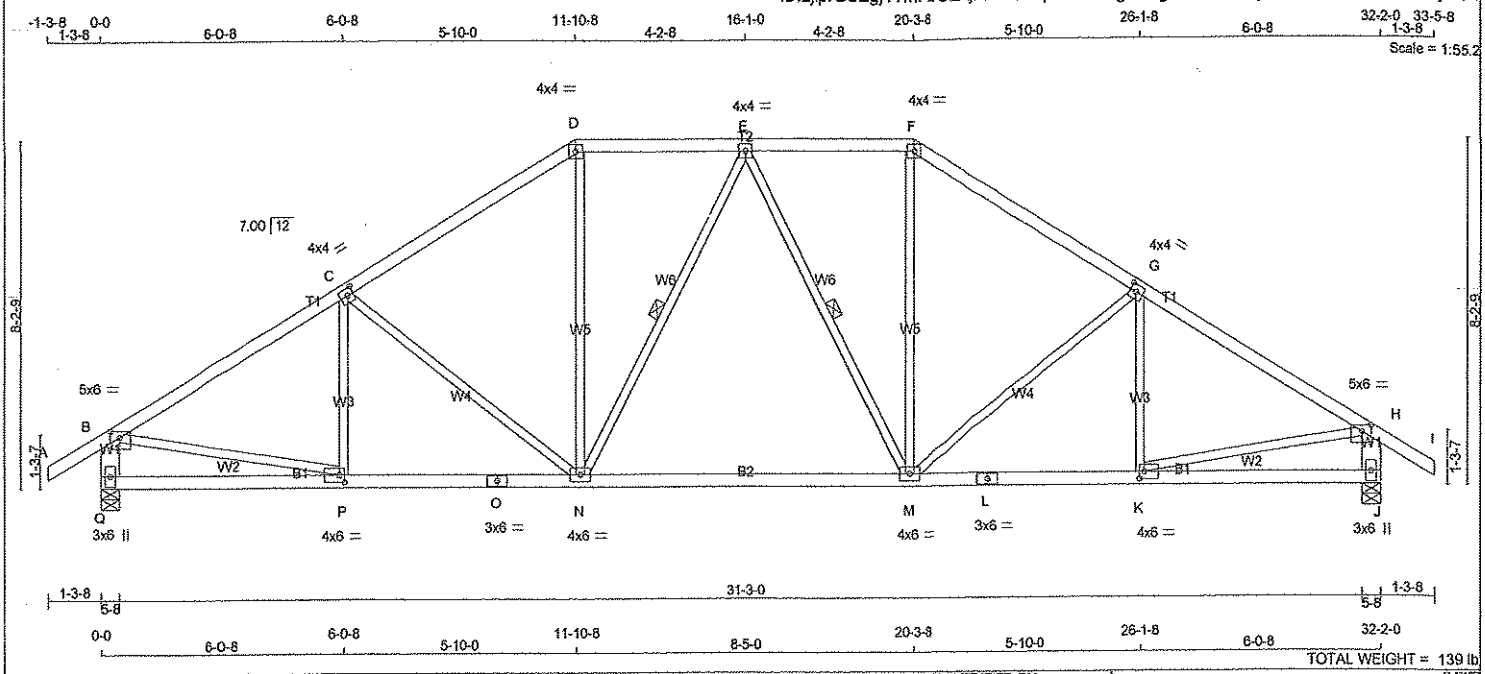


TOTAL WEIGHT = 139 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
Q - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.75	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTW-t	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTW-t	MT20	4.0	4.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
H	TMVW-p	MT20	5.0	6.0	1.75	2.75
J	BMV1+p	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	6.0	2.00	1.50
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BMWWW-t	MT20	4.0	6.0		
O	BS-t	MT20	3.0	6.0		
P	BMWW-t	MT20	4.0	6.0	2.00	1.50
Q	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	1920	0	1920	0	0	5-8	5-8
J	1920	0	1920	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1560	893 / 0	338 / 0	0 / 0	0 / 0	330 / 0	0 / 0
J	1560	893 / 0	338 / 0	0 / 0	0 / 0	330 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

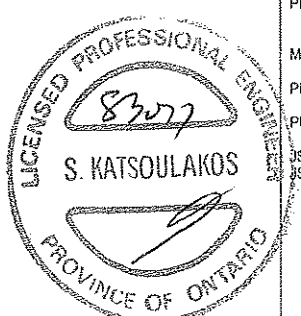
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-N, E-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 29	-84.3	-84.3	0.11 (1)	10.00	P- C	-162 / 140	0.05 (1)
B- C	-2361 / 0	-84.3	-84.3	0.50 (1)	4.00	C- N	-430 / 0	0.47 (1)
C- D	-2025 / 0	-84.3	-84.3	0.45 (1)	4.31	N- D	0 / 680	0.15 (1)
D- E	-1730 / 0	-84.3	-84.3	0.21 (1)	4.87	N- E	-239 / 0	0.13 (1)
E- F	-1730 / 0	-84.3	-84.3	0.21 (1)	4.87	E- M	-239 / 0	0.13 (1)
F- G	-2025 / 0	-84.3	-84.3	0.45 (1)	4.31	M- F	0 / 680	0.15 (1)
G- H	-2361 / 0	-84.3	-84.3	0.50 (1)	4.00	M- G	-430 / 0	0.47 (1)
H- I	0 / 29	-84.3	-84.3	0.11 (1)	10.00	K- G	-162 / 140	0.05 (1)
Q- B	-1848 / 0	0.0	0.0	0.12 (1)	7.35	B- P	0 / 2091	0.47 (1)
J- H	-1848 / 0	0.0	0.0	0.12 (1)	7.35	K- H	0 / 2091	0.47 (1)
Q- P	0 / 0	-28.0	-28.0	0.21 (2)	10.00			
P- O	0 / 2064	-28.0	-28.0	0.55 (2)	10.00			
O- N	0 / 2064	-28.0	-28.0	0.55 (2)	10.00			
N- M	0 / 1841	-28.0	-28.0	0.52 (2)	10.00			
M- L	0 / 2064	-28.0	-28.0	0.55 (2)	10.00			
L- K	0 / 2064	-28.0	-28.0	0.55 (2)	10.00			
K- J	0 / 0	-28.0	-28.0	0.21 (2)	10.00			



DWG NO. TAM 43992-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.07")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (1.07")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.30")

CSI: TC=0.50 (B-C:1) , BC=0.55 (N-P:2) , WB=0.47 (C-N:1) , SSI=0.21 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1657	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SSI GRIP= 0.69 (K) (INPUT = 0.90)
SSI METAL= 0.52 (P) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43994	DRWG NO.
282176	T5	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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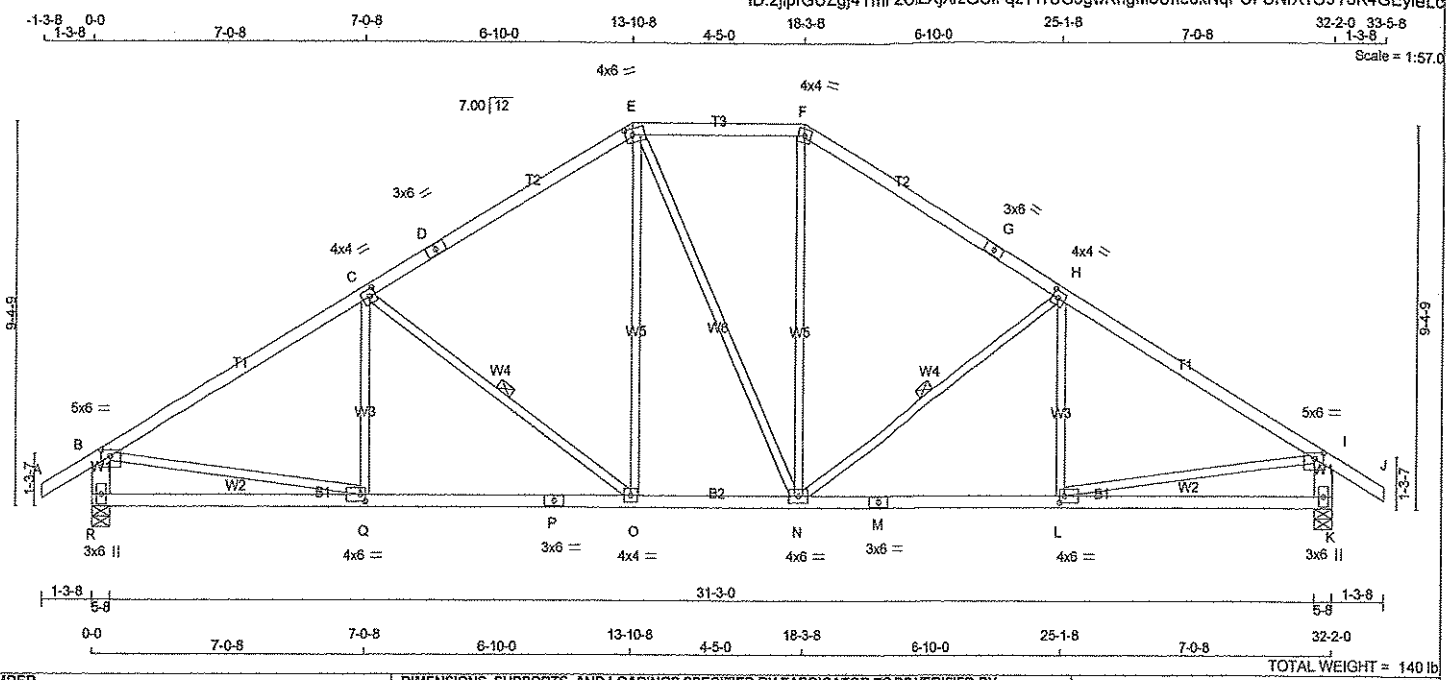

TOTAL WEIGHT = 140 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.75	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	4.0	6.0	1.75	2.25
F	TTW-m	MT20	4.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0	2.00	1.75
I	TMVW-p	MT20	5.0	6.0	1.75	2.75
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	4.0	6.0	2.00	1.50
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BS-t	MT20	3.0	6.0		
Q	BMWW-t	MT20	4.0	6.0	2.00	1.50
R	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	1920	0	1920	0	0	5-8	5-8
K	1920	0	1920	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1560	893 / 0	338 / 0	0 / 0	0 / 0	330 / 0	0 / 0
K	1560	893 / 0	338 / 0	0 / 0	0 / 0	330 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.72 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-O, H-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 29	-84.3	-84.3	0.11 (1)	10.00
B- C	-2371 / 0	-84.3	-84.3	0.72 (1)	3.72
C- D	-1856 / 0	-84.3	-84.3	0.63 (1)	4.21
D- E	-1856 / 0	-84.3	-84.3	0.63 (1)	4.21
E- F	-1578 / 0	-84.3	-84.3	0.24 (1)	5.02
F- G	-1857 / 0	-84.3	-84.3	0.63 (1)	4.21
G- H	-1857 / 0	-84.3	-84.3	0.63 (1)	4.21
H- I	-2370 / 0	-84.3	-84.3	0.72 (1)	3.72
I- J	0 / 29	-84.3	-84.3	0.11 (1)	10.00
R- B	-1841 / 0	0.0	0.0	0.12 (1)	7.37
K- I	-1841 / 0	0.0	0.0	0.12 (1)	7.37
R- Q	0 / 0	-28.0	-28.0	0.35 (3)	10.00
Q- P	0 / 2078	-28.0	-28.0	0.59 (2)	10.00
P- O	0 / 2078	-28.0	-28.0	0.59 (2)	10.00
O- N	0 / 1577	-28.0	-28.0	0.36 (2)	10.00
N- M	0 / 2078	-28.0	-28.0	0.59 (2)	10.00
M- L	0 / 2078	-28.0	-28.0	0.59 (2)	10.00
L- K	0 / 0	-28.0	-28.0	0.35 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-62 / 256	0.06 (3)
C- O	-628 / 0	0.32 (1)
O- E	0 / 515	0.12 (2)
E- N	0 / 2	0.00 (2)
N- F	0 / 518	0.12 (2)
N- H	-627 / 0	0.32 (1)
L- H	-64 / 255	0.06 (3)
B- Q	0 / 2098	0.47 (1)
L- I	0 / 2097	0.47 (1)

DWG NO. TAM 43993-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.07")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (1.07")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.72 (B-C:1) , BC=0.59 (O-Q:2) , WB=0.47 (B-Q:1) , SSI=0.25 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

(SI GRIP= 0.88 (B) (INPUT = 0.90)
(SI METAL= 0.64 (P) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
282176	T6	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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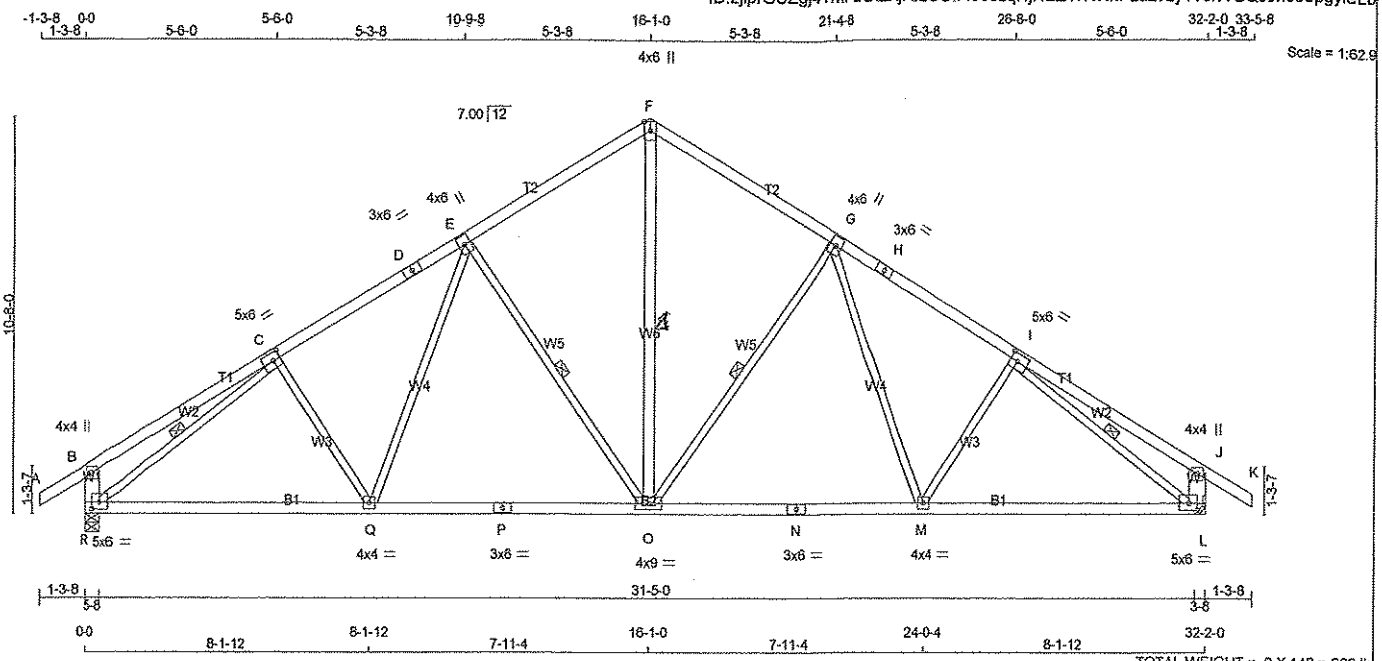

TOTAL WEIGHT = 2 X 142 = 283 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
O - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.50
D	TS-t	MT20	3.0	6.0		
E	TMWW+t	MT20	4.0	6.0		
F	TTW+p	MT20	4.0	6.0	Edge	
G	TMWW+t	MT20	4.0	6.0		
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	5.0	6.0	2.50	2.50
J	TMV+p	MT20	4.0	4.0		
L	BMVW1-t	MT20	5.0	6.0	2.25	2.75
M	BMWW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	6.0		
O	BMWWW-t	MT20	4.0	9.0		
P	BS-t	MT20	3.0	6.0		
Q	BMWW-t	MT20	4.0	4.0		
R	BMVW1-t	MT20	5.0	6.0	2.25	2.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
R	1920	0	1920	0	0	5-8	5-8
L	1920	0	1920	0	0	HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1560	893 / 0	338 / 0	0 / 0	0 / 0	330 / 0	0 / 0
L	1560	893 / 0	338 / 0	0 / 0	0 / 0	330 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.28 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-O, E-O, C-R, I-L. F-01

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 29	-84.3	-84.3	0.11 (1)		10.00
B- C	0 / 21	-84.3	-84.3	0.30 (1)		10.00
C- D	-2238 / 0	-84.3	-84.3	0.34 (1)		4.28
D- E	-2238 / 0	-84.3	-84.3	0.34 (1)		4.28
E- F	-1660 / 0	-84.3	-84.3	0.32 (1)		4.83
F- G	-1660 / 0	-84.3	-84.3	0.32 (1)		4.83
G- H	-2238 / 0	-84.3	-84.3	0.34 (1)		4.28
H- I	-2238 / 0	-84.3	-84.3	0.34 (1)		4.28
I- J	0 / 21	-84.3	-84.3	0.30 (1)		10.00
J- K	0 / 29	-84.3	-84.3	0.11 (1)		10.00
R- B	-301 / 0	0.0	0.0	0.02 (1)		7.81
L- J	-301 / 0	0.0	0.0	0.02 (1)		7.81
R- Q	0 / 1999	-28.0	-28.0	0.68 (2)		10.00
Q- P	0 / 1811	-28.0	-28.0	0.66 (2)		10.00
P- O	0 / 1811	-28.0	-28.0	0.66 (2)		10.00
O- N	0 / 1811	-28.0	-28.0	0.66 (2)		10.00
N- M	0 / 1811	-28.0	-28.0	0.66 (2)		10.00
M- L	0 / 1999	-28.0	-28.0	0.68 (2)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- F	0 / 1290	0.21 (1)
O- G	-671 / 0	0.37 (1)
G- M	0 / 427	0.10 (2)
M- I	-114 / 114	0.05 (1)
E- O	-671 / 0	0.37 (1)
Q- E	0 / 427	0.10 (2)
C- Q	-114 / 114	0.05 (1)
R- C	-2515 / 0	0.75 (1)
I- L	-2515 / 0	0.75 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.07")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.19")
ALLOWABLE DEFL.(TL)= L/360 (1.07")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.32")

CSI: TC=0.34 (G-I:1) , BC=0.68 (Q-R:2) , WB=0.75 (C-R:1) , SSI=0.19 (L-M:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.62 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 43994 17
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
282176	T6CP	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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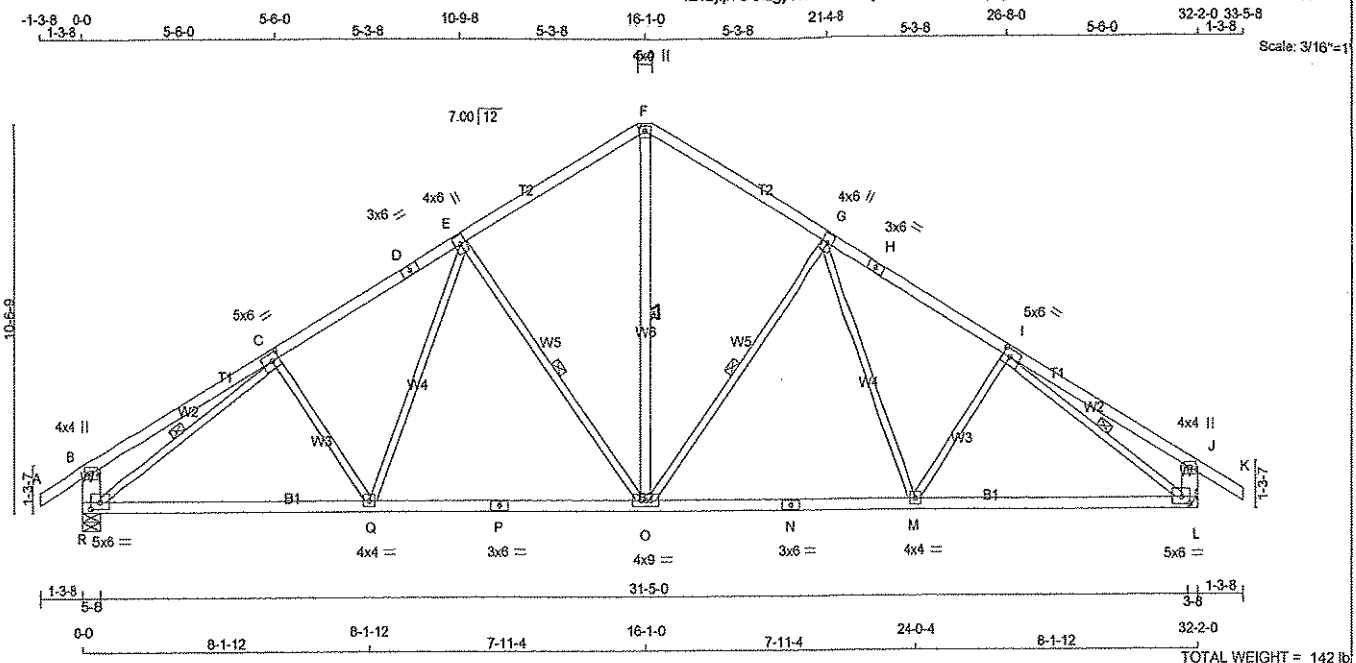

TOTAL WEIGHT = 142 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
O - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.50
D	TS-t	MT20	3.0	6.0		
E	TMWW+t	MT20	4.0	6.0		
F	TTW+p	MT20	4.0	4.0	1.75	2.00
G	TMWW+t	MT20	4.0	6.0		
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	5.0	6.0	2.50	2.50
J	TMV+p	MT20	4.0	4.0		
L	BMVW1-t	MT20	5.0	6.0	2.25	2.75
M	BMWW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	6.0		
O	BMWWW-t	MT20	4.0	9.0		
P	BS-t	MT20	3.0	6.0		
Q	BMWW-t	MT20	4.0	4.0		
R	BMVW1-t	MT20	5.0	6.0	2.25	2.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	1920	0	1920	0	0	5-8	5-8
L	1920	0	1920	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1560	893 / 0	338 / 0	0 / 0	0 / 0	330 / 0	0 / 0
L	1560	893 / 0	338 / 0	0 / 0	0 / 0	330 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.28 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-O, E-O, C-R, I-L. F-O

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 29	-84.3	-84.3	0.11 (1)		10.00
B- C	0 / 21	-84.3	-84.3	0.30 (1)		10.00
C- D	-2238 / 0	-84.3	-84.3	0.34 (1)		4.28
D- E	-2238 / 0	-84.3	-84.3	0.34 (1)		4.28
E- F	-1660 / 0	-84.3	-84.3	0.32 (1)		4.83
F- G	-1660 / 0	-84.3	-84.3	0.32 (1)		4.83
G- H	-2238 / 0	-84.3	-84.3	0.34 (1)		4.28
H- I	-2238 / 0	-84.3	-84.3	0.34 (1)		4.28
I- J	0 / 21	-84.3	-84.3	0.30 (1)		10.00
J- K	0 / 29	-84.3	-84.3	0.11 (1)		10.00
R- B	-301 / 0	0.0	0.0	0.02 (1)		7.81
L- J	-301 / 0	0.0	0.0	0.02 (1)		7.81
R- Q	0 / 1999	-28.0	-28.0	0.68 (2)		10.00
Q- P	0 / 1811	-28.0	-28.0	0.66 (2)		10.00
P- O	0 / 1811	-28.0	-28.0	0.66 (2)		10.00
O- N	0 / 1811	-28.0	-28.0	0.66 (2)		10.00
N- M	0 / 1811	-28.0	-28.0	0.66 (2)		10.00
M- L	0 / 1999	-28.0	-28.0	0.68 (2)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- F	0 / 1290	0.21 (1)
O- G	-671 / 0	0.37 (1)
G- M	0 / 427	0.10 (2)
M- I	-114 / 114	0.05 (1)
E- O	-671 / 0	0.37 (1)
Q- E	0 / 427	0.10 (2)
C- Q	-114 / 114	0.05 (1)
R- C	-2515 / 0	0.75 (1)
I- L	-2515 / 0	0.75 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.07")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.19")
ALLOWABLE DEFL.(TL)= L/360 (1.07")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.32")

CSI: TC=0.34 (G-I:1) , BC=0.68 (Q-R:2) , WB=0.75 (C-R:1) , SSI=0.19 (L-M:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.62 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
83077
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 43995-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
282176	T9	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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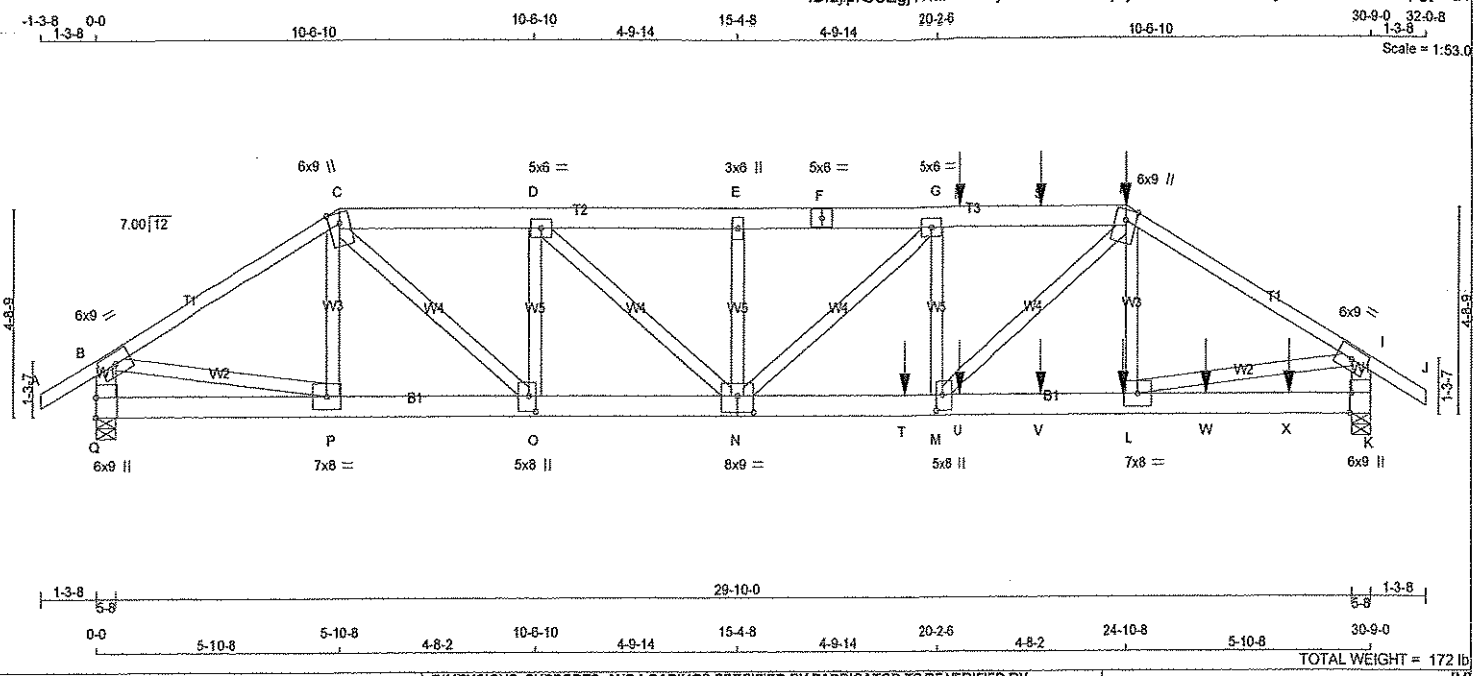


TOTAL WEIGHT = 172 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	1650F 1.5E	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x4	DRY	1650F 1.5E	SPF
Q - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
Q - N	2x6	DRY	1650F 1.5E	SPF
N - K	2x6	DRY	1650F 1.5E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	6.0	9.0		
C	TTWW+m	MT20	6.0	9.0	Edge	
D	TMWW-t	MT20	5.0	6.0		
E	TMW+w	MT20	3.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMWW-t	MT20	5.0	6.0		
H	TTWW+m	MT20	6.0	9.0	Edge	
I	TMVW-t	MT20	6.0	9.0		
K	BMV1+t	MT20	6.0	9.0	Edge	0.50
L	BMWW-t	MT20	7.0	8.0		
M	BMWW+t	MT20	5.0	8.0	4.25	2.00
N	BSWWW-t	MT20	8.0	9.0	4.50	4.50
O	BMWW+t	MT20	5.0	8.0	4.25	2.00
P	BMWW-t	MT20	7.0	8.0		
Q	BMV1+t	MT20	6.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 101.4 lbs FACTORED DOWN AT 20-9-12, AND 101.4 lbs FACTORED DOWN AT 22-9-12, AND 473.2 lbs FACTORED DOWN AT 24-10-8 ON TOP CHORD, AND 1473.7 lbs FACTORED DOWN AT 19-4-8, 70.9 lbs FACTORED DOWN AT 20-9-12, 70.9 lbs FACTORED DOWN AT 22-9-12, 70.9 lbs FACTORED DOWN AT 24-9-12, AND 86.2 lbs FACTORED DOWN AT 26-9-12, AND 86.2 lbs FACTORED DOWN AT 28-9-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	2576	0	2576	0	0	5-8	5-8
K	3476	0	3476	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	2085	1205 / 0	443 / 0	0 / 0	0 / 0	437 / 0	0 / 0
K	2824	1615 / 0	610 / 0	0 / 0	0 / 0	598 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.05 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 29	-84.3	-84.3	0.10 (1)	10.00
B- C	-3374 / 0	-84.3	-84.3	0.71 (1)	3.66
C- D	-4659 / 0	-84.3	-84.3	0.27 (1)	3.82
D- E	-5832 / 0	-84.3	-84.3	0.33 (1)	3.40
E- F	-5832 / 0	-84.3	-84.3	0.34 (1)	3.39
F- G	-5832 / 0	-84.3	-84.3	0.34 (1)	3.39
G- R	-6144 / 0	-84.3	-84.3	0.48 (1)	3.15
R- S	-6144 / 0	-84.3	-84.3	0.48 (1)	3.15
S- H	-6144 / 0	-84.3	-84.3	0.48 (1)	3.15
H- I	-4700 / 0	-84.3	-84.3	0.91 (1)	3.05
I- J	0 / 29	-84.3	-84.3	0.10 (1)	10.00
Q- B	-2504 / 0	0.0	0.0	0.18 (1)	6.50
K- I	-3345 / 0	0.0	0.0	0.24 (1)	5.76
Q- P	0 / 0	-28.0	-28.0	0.07 (2)	10.00
P- O	0 / 2930	-28.0	-28.0	0.28 (1)	10.00
O- N	0 / 4658	-28.0	-28.0	0.47 (1)	10.00
N- T	0 / 6143	-28.0	-28.0	0.86 (1)	10.00
T- M	0 / 6143	-28.0	-28.0	0.86 (1)	10.00
M- U	0 / 4084	-28.0	-28.0	0.53 (1)	10.00
U- V	0 / 4084	-28.0	-28.0	0.53 (1)	10.00
V- L	0 / 4084	-28.0	-28.0	0.53 (1)	10.00
L- W	0 / 0	-28.0	-28.0	0.15 (2)	10.00
W- X	0 / 0	-28.0	-28.0	0.15 (2)	10.00
X- K	0 / 0	-28.0	-28.0	0.15 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- C	-279 / 75	0.06 (1)
L- H	-449 / 183	0.10 (1)
B- P	0 / 2955	0.52 (1)
L- I	0 / 4118	0.73 (1)
M- H	0 / 2793	0.49 (1)
C- O	0 / 2344	0.41 (1)
M- G	-348 / 43	0.07 (1)
O- D	-1510 / 0	0.32 (1)
N- G	-416 / 0	0.21 (1)
D- N	0 / 1565	0.28 (1)
N- E	-375 / 0	0.08 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
H	24-10-8	-473	-473	---	BACK	VERT	TOTAL
L	24-9-12	-41	-71	---	BACK	VERT	TOTAL
R	20-9-12	-101	-101	---	BACK	VERT	TOTAL
S	22-9-12	-101	-101	---	BACK	VERT	TOTAL
T	19-4-8	-1474	-1474	---	BACK	VERT	TOTAL
U	20-9-12	-41	-71	---	BACK	VERT	TOTAL
V	22-9-12	-41	-71	---	BACK	VERT	TOTAL
W	26-9-12	-49	-86	---	BACK	VERT	TOTAL
X	28-9-12	-49	-86	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.02")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.27")
ALLOWABLE DEFL.(TL)= L/360 (1.02")
CALCULATED VERT. DEFL.(TL) = L/ 872 (0.42")

CSI: TC=0.91 (H-I:1) , BC=0.86 (M-N:1) , WB=0.73 (I-L:1) , SSI=0.89 (M-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

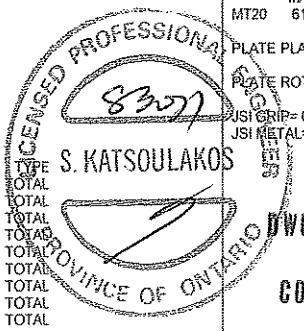
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (I) (INPUT = 0.90)
JSI METAL= 0.88 (N) (INPUT = 1.00)



DWG NO. TAM 43996-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
282176	T10	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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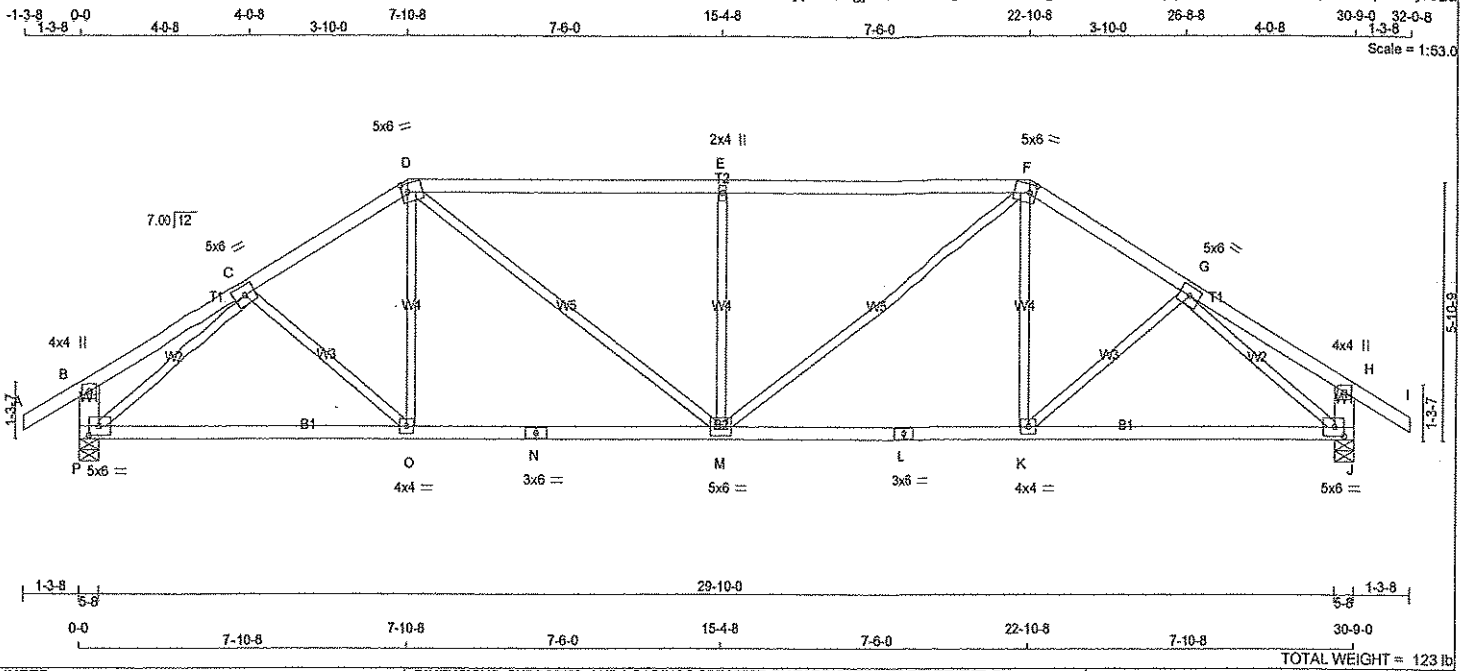


[MI][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
P - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW-m	MT20	5.0	6.0	2.25	1.75
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	1.75
G	TMWW-t	MT20	5.0	6.0		
H	TMV+p	MT20	4.0	4.0		
J	BMVW1-t	MT20	5.0	6.0	2.50	2.75
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	5.0	6.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMVW1-t	MT20	5.0	6.0	2.50	2.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
P	1841	0	1841	0	0	5-8	5-8
J	1841	0	1841	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1495	857 / 0	323 / 0	0 / 0	0 / 0	316 / 0	0 / 0
J	1495	857 / 0	323 / 0	0 / 0	0 / 0	316 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

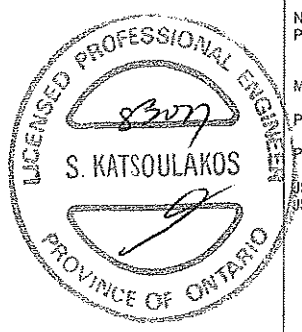
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.47 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO			
A- B	0 / 29	-84.3	-84.3	0.11 (1)	10.00	C- O	0 / 162	0.04 (3)	
B- C	0 / 17	-84.3	-84.3	0.18 (1)	10.00	O- D	0 / 345	0.08 (2)	
C- D	-2153 / 0	-84.3	-84.3	0.20 (1)	4.48	D- M	0 / 805	0.18 (1)	
D- E	-2492 / 0	-84.3	-84.3	0.78 (1)	3.47	M- E	-777 / 0	0.41 (1)	
E- F	-2492 / 0	-84.3	-84.3	0.78 (1)	3.47	M- F	0 / 805	0.18 (1)	
F- G	-2153 / 0	-84.3	-84.3	0.20 (1)	4.48	K- F	0 / 345	0.08 (2)	
G- H	0 / 17	-84.3	-84.3	0.18 (1)	10.00	K- G	0 / 162	0.04 (3)	
H- I	0 / 29	-84.3	-84.3	0.11 (1)	10.00	P- C	-2355 / 0	0.94 (1)	
P- B	-247 / 0	0.0	0.0	0.02 (1)	7.81	G- J	-2355 / 0	0.94 (1)	
J- H	-247 / 0	0.0	0.0	0.02 (1)	7.81				
P- O	0 / 1812	-28.0	-28.0	0.63 (2)	10.00				
O- N	0 / 1847	-28.0	-28.0	0.65 (2)	10.00				
N- M	0 / 1847	-28.0	-28.0	0.65 (2)	10.00				
M- L	0 / 1847	-28.0	-28.0	0.65 (2)	10.00				
L- K	0 / 1847	-28.0	-28.0	0.65 (2)	10.00				
K- J	0 / 1812	-28.0	-28.0	0.63 (2)	10.00				



DWG NO. TAM43997-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.02")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (1.02")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.28")

CSI: TC=0.78 (D-E:1) , BC=0.65 (M-O:2) , WB=0.94 (C-P:1) , SSI=0.31 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

USI GRIP= 0.90 (J) (INPUT = 0.90)
USI METAL= 0.58 (C) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
282176	T11X	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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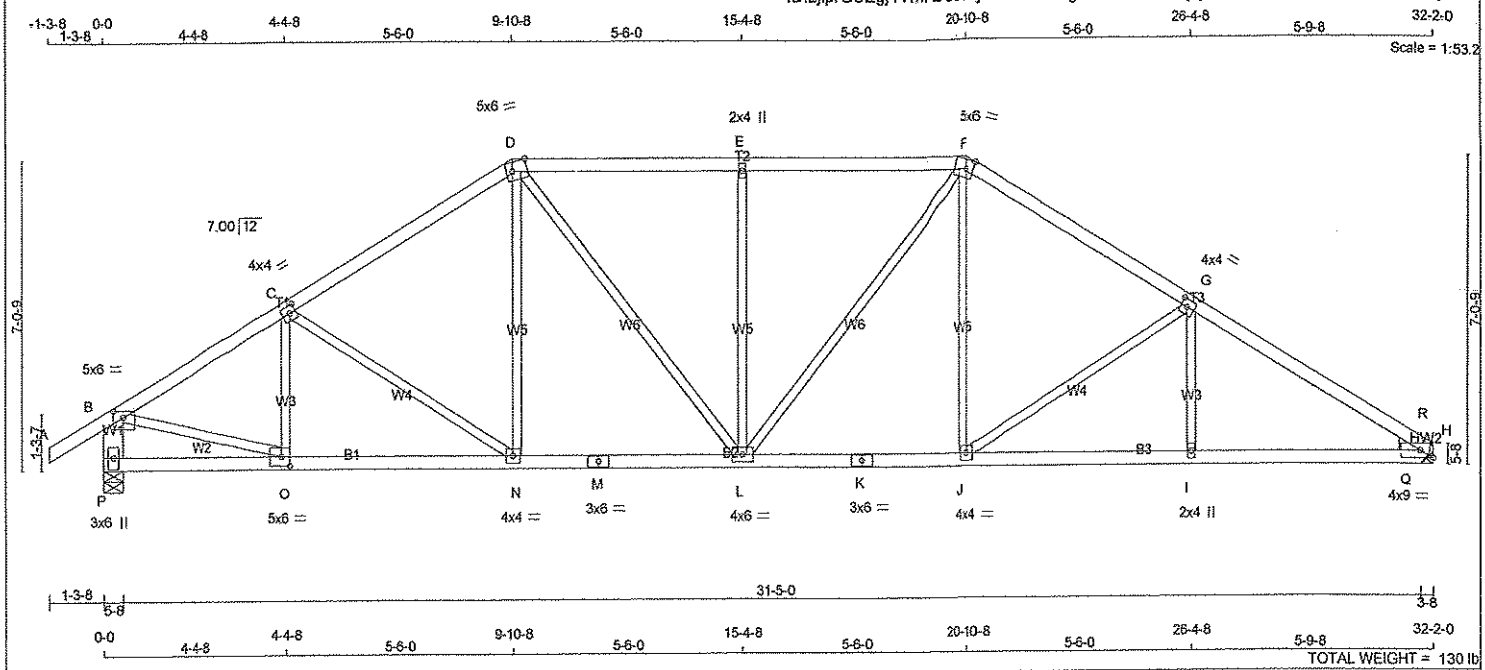

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
P - B	2x6	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW-p	MT20	5.0	6.0	1.75	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	6.0	Edge	4.25
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	Edge	2.25
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
H	TMBH1-t	MT20	4.0	9.0	Edge	3.50
I	BMW+w	MT20	2.0	4.0		
J	BMWW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	4.0	6.0		
M	BS-t	MT20	3.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMWW-t	MT20	5.0	6.0	2.50	2.50
P	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
P	1920	0	1920	0	0	5-8	5-8	
H	1805	0	1805	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8		2x4 R

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1560	893 / 0	338 / 0	0 / 0	0 / 0	330 / 0	0 / 0
H	1482	823 / 0	338 / 0	0 / 0	0 / 0	322 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI	MAX (LC)	MAX. UNBRAC LENGTH
A- B	0 / 29	-84.3	-84.3	0.11 (1)	10.00	
B- C	-2309 / 0	-84.3	-84.3	0.33 (1)	4.21	
C- D	-2155 / 0	-84.3	-84.3	0.38 (1)	4.30	
D- E	-2198 / 0	-84.3	-84.3	0.39 (1)	4.22	
E- F	-2198 / 0	-84.3	-84.3	0.39 (1)	4.22	
F- G	-2301 / 0	-84.3	-84.3	0.39 (1)	4.17	
G- R	-2898 / 0	-84.3	-84.3	0.40 (1)	3.78	
R- H	-3119 / 0	-84.3	-84.3	0.24 (1)	3.77	
P- B	-1870 / 0	0.0	0.0	0.12 (1)	7.32	
P- O	0 / 0	-28.0	-28.0	0.15 (3)	10.00	
O- N	0 / 2019	-28.0	-28.0	0.43 (1)	10.00	
N- M	0 / 1845	-28.0	-28.0	0.40 (1)	10.00	
M- L	0 / 1845	-28.0	-28.0	0.40 (1)	10.00	
L- K	0 / 1974	-28.0	-28.0	0.42 (1)	10.00	
K- J	0 / 1974	-28.0	-28.0	0.42 (1)	10.00	
J- I	0 / 2512	-28.0	-28.0	0.51 (1)	10.00	
I- Q	0 / 2512	-28.0	-28.0	0.47 (1)	10.00	
Q- H	0 / 2512	-28.0	-28.0	0.47 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- C	-304 / 43	0.07 (1)
C- N	-211 / 0	0.17 (1)
N- D	0 / 350	0.08 (2)
D- L	0 / 559	0.13 (1)
L- E	-564 / 0	0.48 (1)
L- F	0 / 354	0.08 (1)
J- F	0 / 520	0.12 (2)
J- G	-645 / 0	0.52 (1)
I- G	0 / 267	0.06 (3)
B- O	0 / 2068	0.47 (1)
Q- R	0 / 281	0.09 (1)

DWG NO. TAM 43998-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.07")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (1.07")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.40 (G-R:1) , BC=0.51 (I-J:1) , WB=0.52 (G-J:1) , SSI=0.22 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (O) (INPUT = 0.90)
JSI METAL= 0.57 (K) (INPUT = 1.00)

[M][F]

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
282176	T12X	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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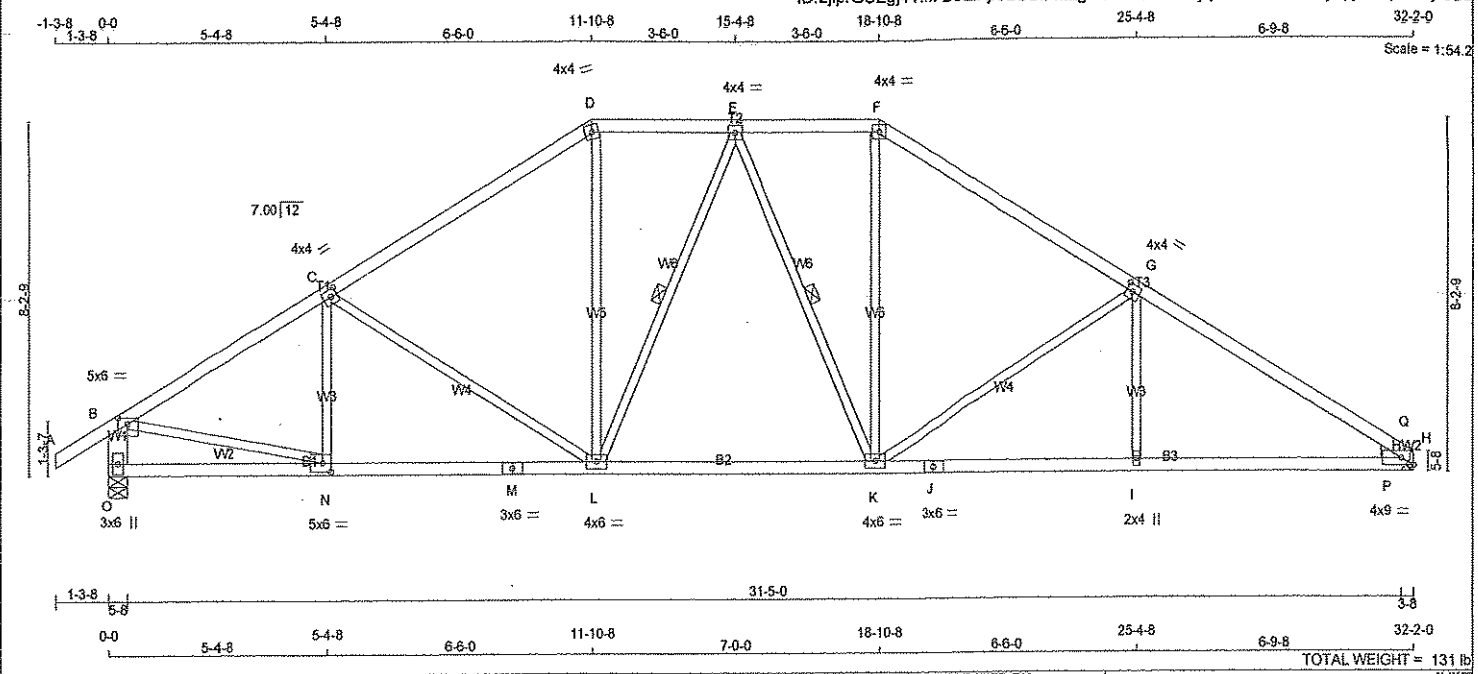


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
O - B	2x6	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.75	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTW-l	MT20	4.0	4.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
H	TMBH1-l	MT20	4.0	9.0	Edge	3.50
I	BMW+w	MT20	2.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMWWW-t	MT20	4.0	6.0		
M	BS-t	MT20	3.0	6.0		
N	BMWW-t	MT20	5.0	6.0	2.50	2.50
O	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG	HEEL WEDGE
O	1920	0	1920	0	0	5-8	5-8	
H	1805	0	1805	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8		2x4 R

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1560	893 / 0	338 / 0	0 / 0	0 / 0	330 / 0	0 / 0
H	1482	823 / 0	338 / 0	0 / 0	0 / 0	322 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.64 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-L, E-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

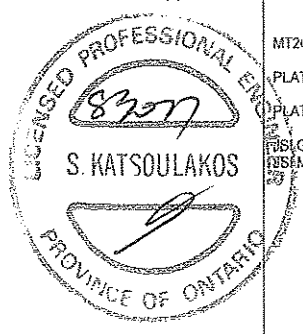
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 29	-84.3	-84.3	0.11 (1)	10.00
B- C	-2368 / 0	-84.3	-84.3	0.48 (1)	3.99
C- D	-2020 / 0	-84.3	-84.3	0.53 (1)	4.24
D- E	-1728 / 0	-84.3	-84.3	0.15 (1)	4.95
E- F	-1795 / 0	-84.3	-84.3	0.15 (1)	4.87
F- G	-2098 / 0	-84.3	-84.3	0.54 (1)	4.16
G- Q	-2837 / 0	-84.3	-84.3	0.58 (1)	3.64
Q- H	-3047 / 0	-84.3	-84.3	0.25 (1)	3.80
O- B	-1860 / 0	0.0	0.0	0.12 (1)	7.34
O- N	0 / 0	-28.0	-28.0	0.22 (3)	10.00
N- M	0 / 2074	-28.0	-28.0	0.50 (2)	10.00
M- L	0 / 2074	-28.0	-28.0	0.50 (2)	10.00
L- K	0 / 1835	-28.0	-28.0	0.47 (2)	10.00
K- J	0 / 2466	-28.0	-28.0	0.54 (2)	10.00
J- I	0 / 2466	-28.0	-28.0	0.54 (2)	10.00
I- P	0 / 2466	-28.0	-28.0	0.52 (1)	10.00
P- H	0 / 2466	-28.0	-28.0	0.42 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- C	-197 / 133	0.06 (1)
C- L	-419 / 0	0.52 (1)
L- D	0 / 685	0.15 (1)
L- E	-273 / 0	0.14 (1)
E- K	-101 / 32	0.05 (1)
K- F	0 / 705	0.16 (1)
K- G	-801 / 0	0.99 (1)
I- G	0 / 325	0.07 (2)
B- N	0 / 2108	0.47 (1)
P- Q	0 / 314	0.09 (1)



DWG NO. TAM 43999-17
STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.07")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (1.07")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.23")

CSI: TC=0.58 (G-Q:1) , BC=0.54 (I-K:2) , WB=0.99 (G-K:1) , SSI=0.22 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SSI GRIP= 0.90 (D) (INPUT = 0.90)
SSMETAL= 0.62 (J) (INPUT = 1.00)

[M][F]

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
282176	T13X	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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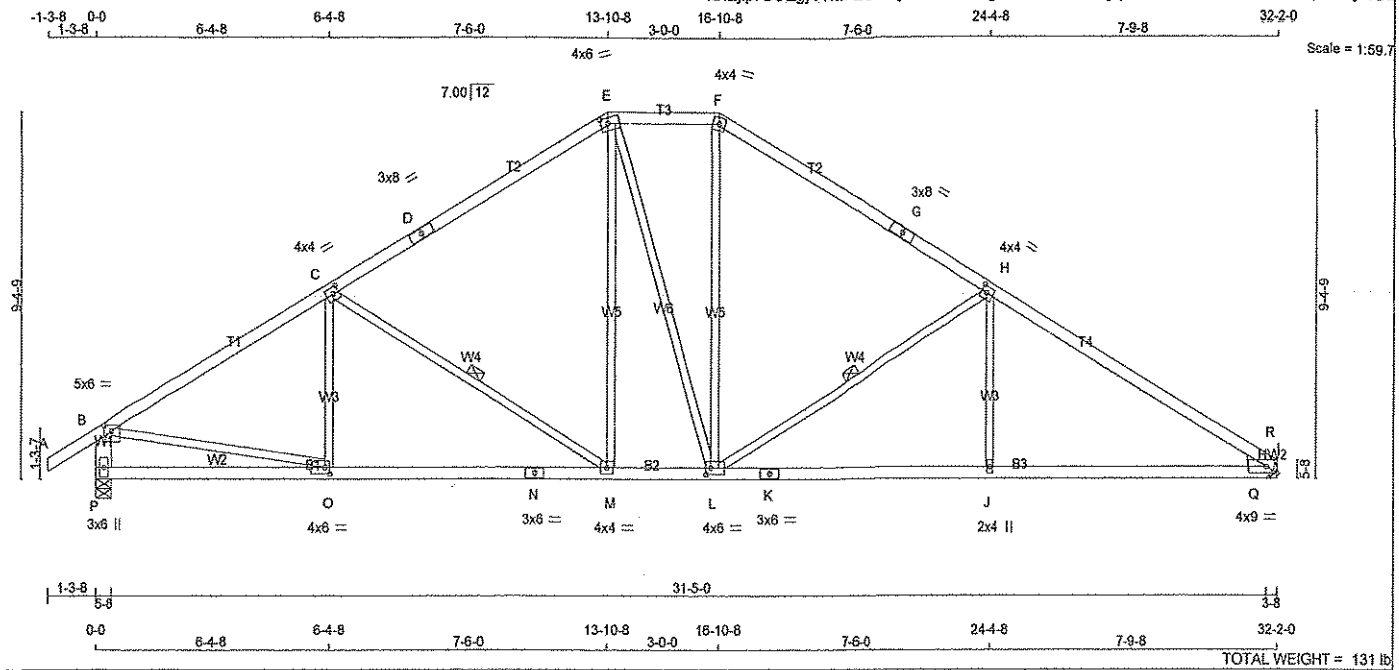

TOTAL WEIGHT = 131 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
P - B	2x6	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.75	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	8.0		
E	TTWW-m	MT20	4.0	6.0	1.75	2.25
F	TTW-m	MT20	4.0	4.0		
G	TS-t	MT20	3.0	8.0		
H	TMWW-t	MT20	4.0	4.0	2.00	1.75
I	TMBH1-I	MT20	4.0	9.0	Edge	3.50
J	BMW+w	MT20	2.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	4.0	6.0	2.00	1.50
M	BMWW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	6.0	2.00	1.50
P	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
P	1920	0	1920	0	0	5-8	5-8	
I	1805	0	1805	0	0	HANGER BY OTHERS		2x4 R
						MIN. SEAT SIZE: 3-8		

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1560	893 / 0	338 / 0	0 / 0	0 / 0	330 / 0	0 / 0
I	1462	823 / 0	338 / 0	0 / 0	0 / 0	322 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.41 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-M, H-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 29	-84.3	-84.3	0.11 (1)	10.00
B- C	-2394 / 0	-84.3	-84.3	0.67 (1)	3.72
C- D	-1857 / 0	-84.3	-84.3	0.72 (1)	4.09
D- E	-1857 / 0	-84.3	-84.3	0.72 (1)	4.09
E- F	-1605 / 0	-84.3	-84.3	0.12 (1)	5.11
F- G	-1886 / 0	-84.3	-84.3	0.73 (1)	4.04
G- H	-1886 / 0	-84.3	-84.3	0.73 (1)	4.04
H- R	-2773 / 0	-84.3	-84.3	0.82 (1)	3.41
R- I	-2984 / 0	-84.3	-84.3	0.26 (1)	3.84
P- B	-1852 / 0	0.0	0.0	0.12 (1)	7.35
P- O	0 / 0	-28.0	-28.0	0.34 (3)	10.00
O- N	0 / 2102	-28.0	-28.0	0.58 (2)	10.00
N- M	0 / 2102	-28.0	-28.0	0.58 (2)	10.00
M- L	0 / 1580	-28.0	-28.0	0.40 (2)	10.00
L- K	0 / 2416	-28.0	-28.0	0.64 (2)	10.00
K- J	0 / 2416	-28.0	-28.0	0.64 (2)	10.00
J- Q	0 / 2416	-28.0	-28.0	0.64 (2)	10.00
Q- I	0 / 2416	-28.0	-28.0	0.42 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- C	-99 / 235	0.05 (3)
C- M	-620 / 0	0.34 (1)
M- E	0 / 471	0.11 (2)
E- L	0 / 83	0.02 (1)
L- F	0 / 561	0.13 (1)
L- H	-961 / 0	0.53 (1)
J- H	0 / 403	0.09 (2)
B- O	0 / 2126	0.48 (1)
Q- R	0 / 344	0.09 (1)

DWG NO. TAM 42000-17
STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.07")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (1.07")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.27")

CSI: TC=0.62 (H-R:1) , BC=0.64 (J-L:2) , WB=0.53 (H-L:1) , SSI=0.26 (F-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

USI GRIP= 0.89 (B) (INPUT = 0.90)
USI METAL= 0.66 (N) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
282176	T14X	5	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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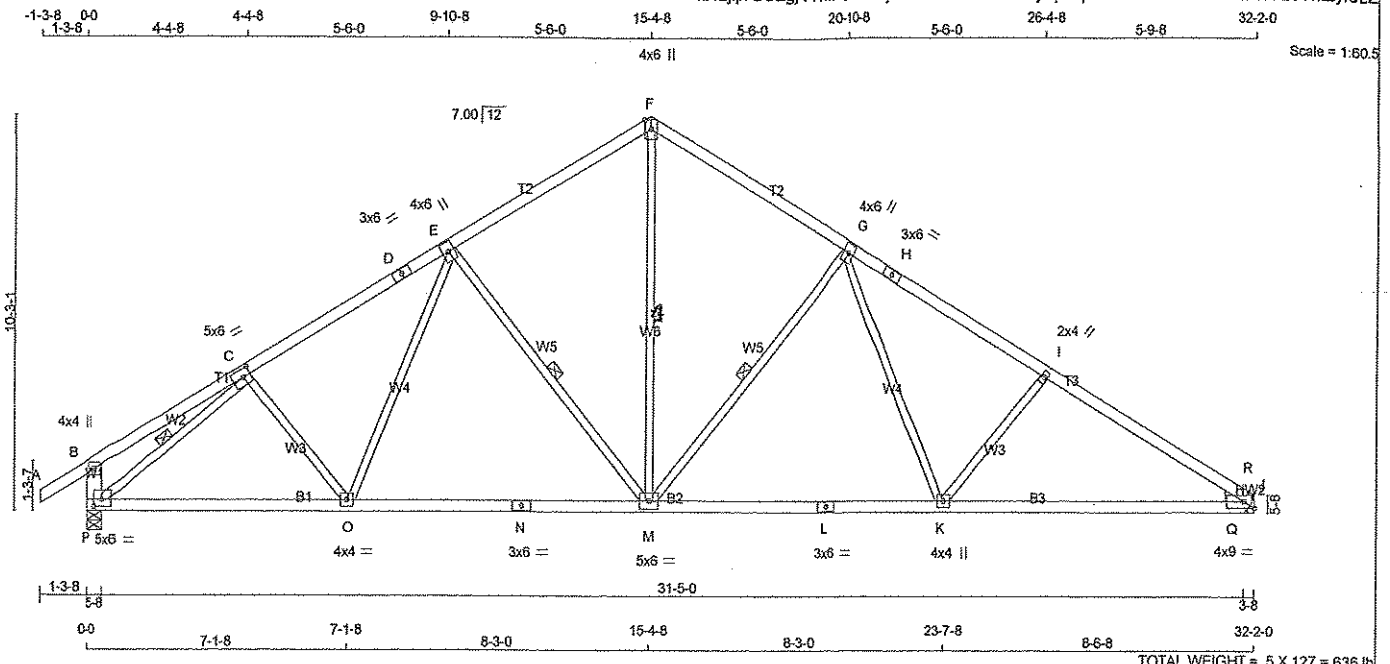

TOTAL WEIGHT = 5 X 127 = 636 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
P - B	2x6	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.50
D	TS-t	MT20	3.0	6.0		
E	TMWW+t	MT20	4.0	6.0		
F	TTW+p	MT20	4.0	6.0	Edge	
G	TMWW+t	MT20	4.0	6.0		
H	TS-t	MT20	3.0	6.0		
I	TMW+w	MT20	2.0	4.0		
J	TMBH1-t	MT20	4.0	9.0	Edge	3.50
K	BMWW+t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	5.0	6.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMVW1-t	MT20	5.0	6.0	2.25	2.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
P	1920	0	1920	0	0	5-8	5-8	
J	1805	0	1805	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8		2x4 R

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1560	893 / 0	338 / 0	0 / 0	0 / 0	330 / 0	0 / 0
J	1482	823 / 0	338 / 0	0 / 0	0 / 0	322 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.83 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-M, G-M, C-P. F-M/

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 29	-84.3	-84.3	1	0.11 (1)	10.00
B- C	0 / 20	-84.3	-84.3	1	0.23 (1)	10.00
C- D	-2259 / 0	-84.3	-84.3	1	0.39 (1)	4.22
D- E	-2259 / 0	-84.3	-84.3	1	0.39 (1)	4.22
E- F	-1730 / 0	-84.3	-84.3	1	0.36 (1)	4.72
F- G	-1729 / 0	-84.3	-84.3	1	0.35 (1)	4.72
G- H	-2558 / 0	-84.3	-84.3	1	0.41 (1)	3.99
H- I	-2558 / 0	-84.3	-84.3	1	0.41 (1)	3.99
I- R	-2849 / 0	-84.3	-84.3	1	0.35 (1)	3.88
R- J	-3108 / 0	-84.3	-84.3	1	0.20 (1)	3.83
P- B	-256 / 0	0.0	0.0	1	0.02 (1)	7.81
P- O	0 / 1954	-28.0	-28.0	2	0.61 (2)	10.00
O- N	0 / 1868	-28.0	-28.0	2	0.60 (2)	10.00
N- M	0 / 1868	-28.0	-28.0	2	0.60 (2)	10.00
M- L	0 / 1998	-28.0	-28.0	2	0.62 (2)	10.00
L- K	0 / 1998	-28.0	-28.0	2	0.62 (2)	10.00
K- Q	0 / 2469	-28.0	-28.0	2	0.67 (2)	10.00
Q- J	0 / 2469	-28.0	-28.0	2	0.49 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- O	0 / 156	0.04 (3)
O- E	0 / 343	0.08 (2)
E- M	-622 / 0	0.32 (1)
M- F	0 / 1350	0.30 (1)
M- G	-827 / 0	0.43 (1)
G- K	0 / 596	0.13 (2)
K- I	-399 / 0	0.14 (1)
P- C	-2516 / 0	0.55 (1)
Q- R	0 / 450	0.00 (1)

DWG NO. TAM 4403/-17
STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.07")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (1.07")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.28")

CSI: TC=0.41 (G-I:1) , BC=0.67 (K-Q:2) , WB=0.55 (C-P:1) , SSI=0.21 (J-Q:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

USI GRIP= 0.88 (P) (INPUT = 0.90)
USI METAL= 0.66 (L) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
282176	T15	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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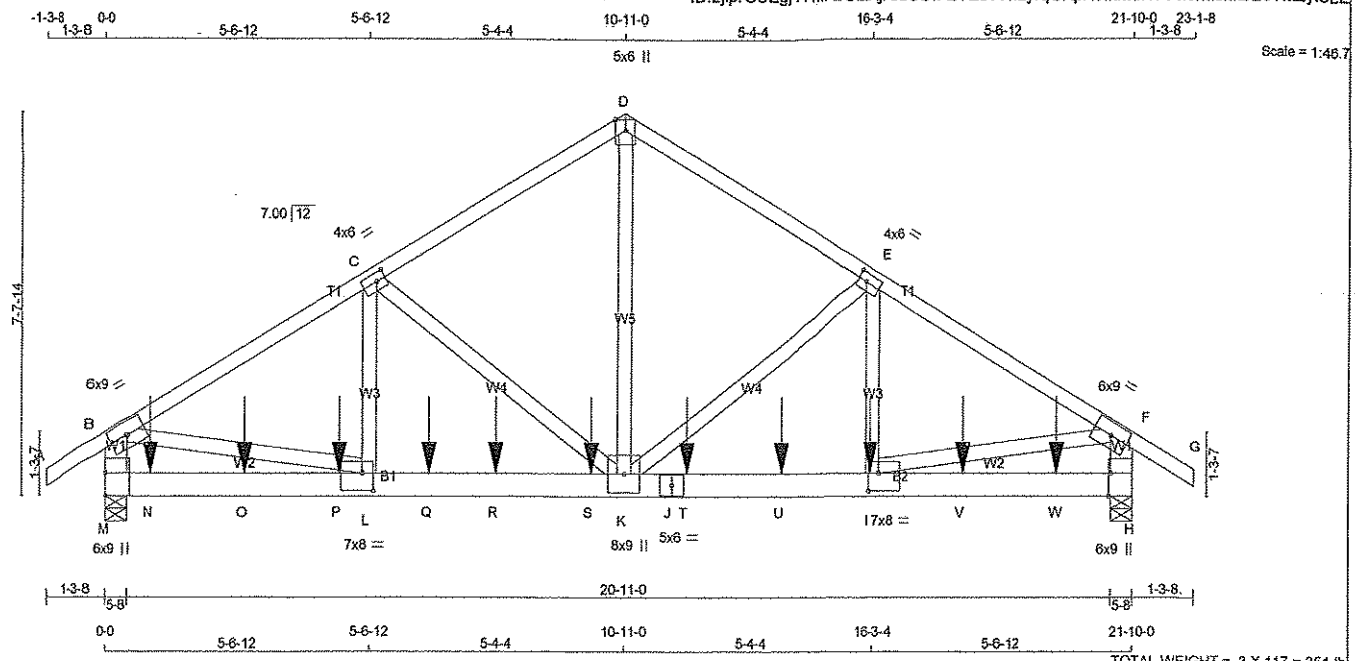

TOTAL WEIGHT = 3 X 117 = 351 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
M - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
M - J	2x6	DRY	1650F 1.5E	SPF
J - H	2x6	DRY	1650F 1.5E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	TOP
D- G	1	12	TOP
M- B	2	12	TOP
H- F	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
M- J	2	7	SIDE(1421.0)
J- H	2	7	SIDE(609.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	6.0	9.0		
C	TMWW-t	MT20	4.0	6.0	2.00	2.25
D	TTW+p	MT20	5.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0	2.00	2.25
F	TMVW-t	MT20	6.0	9.0		
H	BMV1+t	MT20	6.0	9.0	Edge	0.50
I	BMWW-t	MT20	7.0	8.0	4.25	2.75
J	BS-t	MT20	5.0	6.0		
K	BMWWW+t	MT20	8.0	9.0		
L	BMWW-t	MT20	7.0	8.0	4.25	2.75
M	BMV1+t	MT20	6.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
M	11807	0	11807	0	0	5-8	5-8
H	10771	0	10771	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	9567	5517 / 0	2042 / 0	0 / 0	0 / 0	2008 / 0	0 / 0
H	8757	5002 / 0	1902 / 0	0 / 0	0 / 0	1854 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.96 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 29	-84.3	-84.3	0.04 (1)	10.00	K- D	0 / 9996	0.54 (1)
B- C	-14398 / 0	-84.3	-84.3	0.62 (1)	2.96	K- E	-3832 / 0	0.74 (1)
C- D	-10315 / 0	-84.3	-84.3	0.34 (1)	3.62	I- E	0 / 3579	0.19 (1)
D- E	-10315 / 0	-84.3	-84.3	0.34 (1)	3.62	C- K	-4495 / 0	0.87 (1)
E- F	-13791 / 0	-84.3	-84.3	0.58 (1)	3.05	L- C	0 / 4292	0.23 (1)
F- G	0 / 29	-84.3	-84.3	0.04 (1)	10.00	B- L	0 / 12610	0.68 (1)
M- B	-9567 / 0	0.0	0.0	0.21 (1)	5.93	I- F	0 / 12079	0.65 (1)
H- F	-9177 / 0	0.0	0.0	0.20 (1)	6.03			
M- N	0 / 0	-28.0	-28.0	0.44 (1)	10.00			
N- O	0 / 0	-28.0	-28.0	0.44 (1)	10.00			
O- P	0 / 0	-28.0	-28.0	0.44 (1)	10.00			
P- L	0 / 0	-28.0	-28.0	0.44 (1)	10.00			
L- Q	0 / 12452	-28.0	-28.0	0.75 (1)	10.00			
Q- R	0 / 12452	-28.0	-28.0	0.75 (1)	10.00			
R- S	0 / 12452	-28.0	-28.0	0.75 (1)	10.00			
S- K	0 / 12452	-28.0	-28.0	0.75 (1)	10.00			
K- J	0 / 11928	-28.0	-28.0	0.63 (1)	10.00			
J- T	0 / 11928	-28.0	-28.0	0.63 (1)	10.00			
T- U	0 / 11928	-28.0	-28.0	0.63 (1)	10.00			
U- I	0 / 11928	-28.0	-28.0	0.63 (1)	10.00			
I- V	0 / 0	-28.0	-28.0	0.39 (1)	10.00			
V- W	0 / 0	-28.0	-28.0	0.39 (1)	10.00			
W- H	0 / 0	-28.0	-28.0	0.39 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
I	16-2-12	-1777	-1777	---	BACK	VERT	TOTAL
N	11-4	-1893	-1893	---	BACK	VERT	TOTAL
O	2-11-4	-1892	-1892	---	BACK	VERT	TOTAL
P	4-11-4	-1892	-1892	---	BACK	VERT	TOTAL
Q	6-9-12	-1777	-1777	---	BACK	VERT	TOTAL
R	8-2-12	-1777	-1777	---	BACK	VERT	TOTAL
S	10-2-12	-1777	-1777	---	BACK	VERT	TOTAL
T	12-2-12	-1777	-1777	---	BACK	VERT	TOTAL
U	14-2-12	-1777	-1777	---	BACK	VERT	TOTAL
V	18-2-12	-1777	-1777	---	BACK	VERT	TOTAL
W	20-2-12	-1777	-1777	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.26")

CSI: TC=0.62 (B-C:1) , BC=0.75 (K-L:1) , WB=0.87 (C-K:1) , SSI=0.72 (L-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI-GRIP= 0.90 (D) (INPUT = 0.90)
JSI METAL= 0.81 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM 4400247
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
282176	T15	1	3	TRUSS DESC.	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1893.3 lbs FACTORED DOWN AT 11-4, 1892.4 lbs FACTORED DOWN AT 2-11-4, 1892.4 lbs FACTORED DOWN AT 4-11-4, 1777.4 lbs FACTORED DOWN AT 6-9-12, 1777.4 lbs FACTORED DOWN AT 8-2-12, 1777.4 lbs FACTORED DOWN AT 10-2-12, 1777.4 lbs FACTORED DOWN AT 12-2-12, 1777.4 lbs FACTORED DOWN AT 14-2-12, 1777.4 lbs FACTORED DOWN AT 16-2-12, AND 1777.4 lbs FACTORED DOWN AT 18-2-12, AND 1777.4 lbs FACTORED DOWN AT 20-2-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM44022-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
282176	G15	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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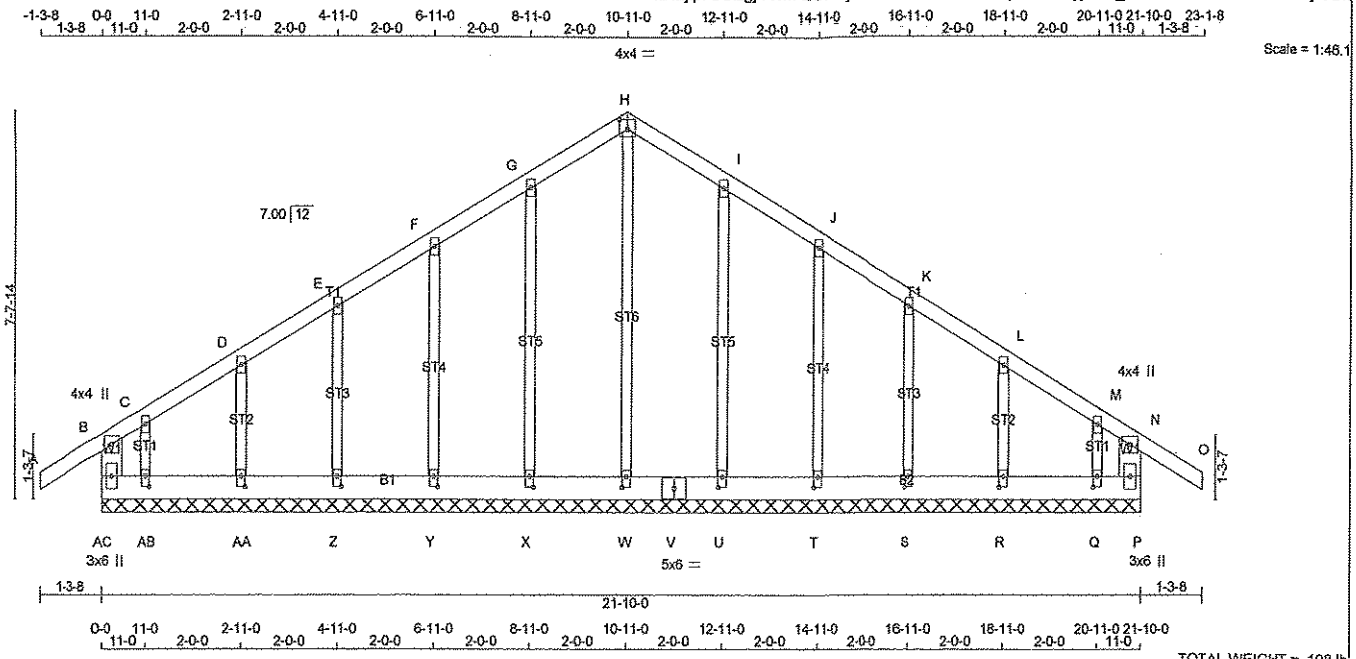

TOTAL WEIGHT = 108 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - H	2x4	DRY	No.2	SPF
H - O	2x4	DRY	No.2	SPF
AC- B	2x6	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
AC- V	2x6	DRY	1650F 1.5E	SPF
V - P	2x6	DRY	1650F 1.5E	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C, D, E, F, G, I, J, K, L, M						
C	TMW+w	MT20	2.0	4.0		
H	TTW-p	MT20	4.0	4.0	2.25	2.00
N	TMV+p	MT20	4.0	4.0		
P	BMV1+p	MT20	3.0	6.0		
Q, R, S, T, U, W, X, Y, Z, AA, AB						
Q	BMW1+w	MT20	2.0	4.0	2.50	1.00
V	BS-I	MT20	5.0	6.0		
AC	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 29	-84.3	-84.3	0.11 (1)	10.00	
B- C	-27 / 0	-84.3	-84.3	0.06 (1)	6.25	
C- D	-3 / 0	-84.3	-84.3	0.04 (1)	10.00	
D- E	-2 / 0	-84.3	-84.3	0.04 (1)	10.00	
E- F	0 / 3	-84.3	-84.3	0.04 (1)	10.00	
F- G	0 / 6	-84.3	-84.3	0.04 (1)	10.00	
G- H	0 / 8	-84.3	-84.3	0.04 (1)	10.00	
H- I	0 / 8	-84.3	-84.3	0.04 (1)	10.00	
I- J	0 / 6	-84.3	-84.3	0.04 (1)	10.00	
J- K	0 / 3	-84.3	-84.3	0.04 (1)	10.00	
K- L	-2 / 0	-84.3	-84.3	0.04 (1)	10.00	
L- M	-3 / 0	-84.3	-84.3	0.04 (1)	10.00	
M- N	-27 / 0	-84.3	-84.3	0.06 (1)	6.25	
N- O	0 / 29	-84.3	-84.3	0.11 (1)	10.00	
AC- B	-187 / 0	0.0	0.0	0.03 (1)	7.81	
P- N	-187 / 0	0.0	0.0	0.03 (1)	7.81	
AC-AB	0 / 10	-28.0	-28.0	0.02 (1)	10.00	
AB-AA	0 / 6	-28.0	-28.0	0.01 (2)	10.00	
AA- Z	0 / 1	-28.0	-28.0	0.01 (2)	10.00	
Z- Y	-2 / 0	-28.0	-28.0	0.01 (2)	10.00	
Y- X	-5 / 0	-28.0	-28.0	0.01 (2)	10.00	
X- W	-7 / 0	-28.0	-28.0	0.01 (2)	10.00	
W- V	-7 / 0	-28.0	-28.0	0.01 (2)	10.00	
V- U	-7 / 0	-28.0	-28.0	0.01 (2)	10.00	
U- T	-5 / 0	-28.0	-28.0	0.01 (2)	10.00	
T- S	-2 / 0	-28.0	-28.0	0.01 (2)	10.00	
S- R	0 / 1	-28.0	-28.0	0.01 (2)	10.00	
R- Q	0 / 6	-28.0	-28.0	0.01 (2)	10.00	
Q- P	0 / 10	-28.0	-28.0	0.02 (1)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
W- H	-182 / 0	0.18 (1)
X- G	-172 / 0	0.11 (1)
Y- F	-166 / 0	0.07 (1)
Z- E	-165 / 0	0.04 (1)
AA- D	-175 / 0	0.03 (1)
AB- C	-80 / 0	0.01 (1)
U- I	-172 / 0	0.11 (1)
T- J	-166 / 0	0.07 (1)
S- K	-165 / 0	0.04 (1)
R- L	-175 / 0	0.03 (1)
Q- M	-80 / 0	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11 (A-B:1) , BC=0.02 (AB-AC:1) , WB=0.18 (H-W:1) , SSI=0.08 (N-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.51 (U) (INPUT = 0.90)
SI METAL= 0.05 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
83077
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM44011-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
282176	G16	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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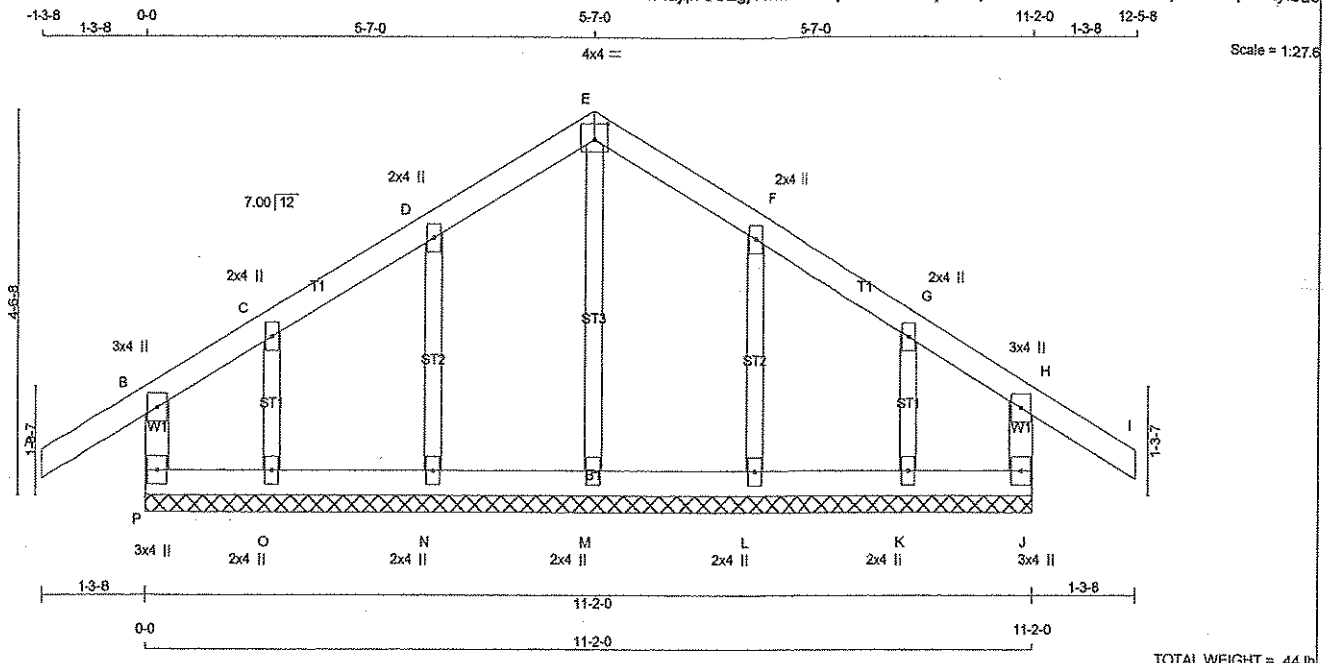


TOTAL WEIGHT = 44 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
P - B	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C, D, F, G						
C	TMW+w	MT20	2.0	4.0		
E	TTW-p	MT20	4.0	4.0	2.25	2.00
H	TMV+p	MT20	3.0	4.0		
J	BMV1+p	MT20	3.0	4.0		
K, L, M, N, O						
K	BMW1+w	MT20	2.0	4.0		
P	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- B	-187 / 0	0.0	0.0	0.04 (1)	7.81	M- E	-207 / 0	0.06 (1)
A- B	0 / 29	-84.3	-84.3	0.11 (1)	10.00	N- D	-181 / 0	0.04 (1)
B- C	-3 / 17	-84.3	-84.3	0.07 (1)	10.00	O- C	-114 / 0	0.02 (1)
C- D	-1 / 38	-84.3	-84.3	0.05 (1)	10.00	L- F	-181 / 0	0.04 (1)
D- E	-1 / 37	-84.3	-84.3	0.05 (1)	10.00	K- G	-114 / 0	0.02 (1)
E- F	-1 / 37	-84.3	-84.3	0.05 (1)	10.00			
F- G	-1 / 38	-84.3	-84.3	0.05 (1)	10.00			
G- H	-3 / 17	-84.3	-84.3	0.07 (1)	10.00			
H- I	0 / 29	-84.3	-84.3	0.11 (1)	10.00			
J- H	-187 / 0	0.0	0.0	0.04 (1)	7.81			
P- O	-24 / 2	-28.0	-28.0	0.02 (3)	6.25			
O- N	-29 / 2	-28.0	-28.0	0.02 (3)	6.25			
N- M	-33 / 1	-28.0	-28.0	0.02 (3)	6.25			
M- L	-33 / 1	-28.0	-28.0	0.02 (3)	6.25			
L- K	-29 / 2	-28.0	-28.0	0.02 (3)	6.25			
K- J	-24 / 2	-28.0	-28.0	0.02 (3)	6.25			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11 (A-B:1) , BC=0.02 (N-O:3) , WB=0.06 (E-M:1) , SSI=0.08 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

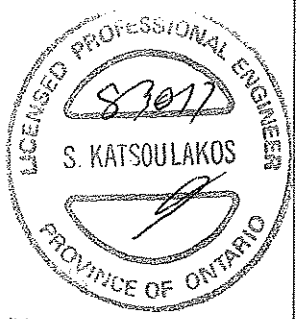
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.16 (E) (INPUT = 0.90)
JSI METAL= 0.05 (E) (INPUT = 1.00)



DWG NO. TAM44012-17

STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
282176	T17	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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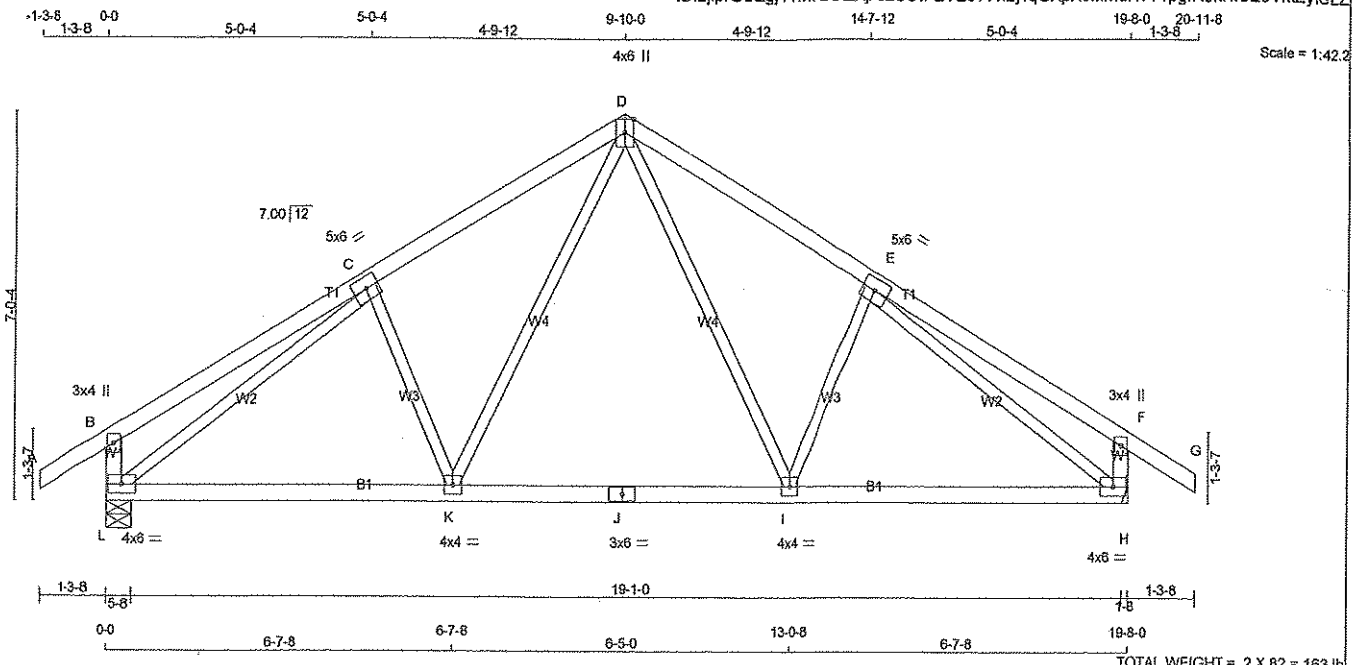


TOTAL WEIGHT = 2 X 82 = 163 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	5.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
L	1219	0	1219	0	0	IN-SX	IN-SX
H	1219	0	1219	0	0	5-8	5-8
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	984	573 / 0	207 / 0	0 / 0	0 / 0	205 / 0	0 / 0
H	984	573 / 0	207 / 0	0 / 0	0 / 0	205 / 0	0 / 0

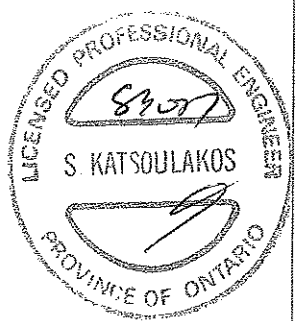
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.55 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 29	-84.3	-84.3	0.11 (1)	10.00	D- I	0 / 476	0.11 (1)	
B- C	0 / 24	-84.3	-84.3	0.33 (1)	10.00	I- E	-252 / 53	0.08 (1)	
C- D	-1193 / 0	-84.3	-84.3	0.27 (1)	5.55	K- D	0 / 476	0.11 (1)	
D- E	-1193 / 0	-84.3	-84.3	0.27 (1)	5.55	C- K	-252 / 53	0.08 (1)	
E- F	0 / 24	-84.3	-84.3	0.33 (1)	10.00	L- C	-1412 / 0	0.94 (1)	
F- G	0 / 29	-84.3	-84.3	0.11 (1)	10.00	E- H	-1412 / 0	0.94 (1)	
L- B	-273 / 0	0.0	0.0	0.03 (1)	7.81				
H- F	-273 / 0	0.0	0.0	0.03 (1)	7.81				
L- K	0 / 1112	-28.0	-28.0	0.40 (2)	10.00				
K- J	0 / 804	-28.0	-28.0	0.37 (2)	10.00				
J- I	0 / 804	-28.0	-28.0	0.37 (2)	10.00				
I- H	0 / 1112	-28.0	-28.0	0.40 (2)	10.00				



DWG NO. TAM4403-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.33 (E-F:1) , BC=0.40 (H-I:2) , WB=0.94 (E-H:1) , SSI=0.17 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (H) (INPUT = 0.90)
JSI METAL= 0.36 (C) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
282176	G17	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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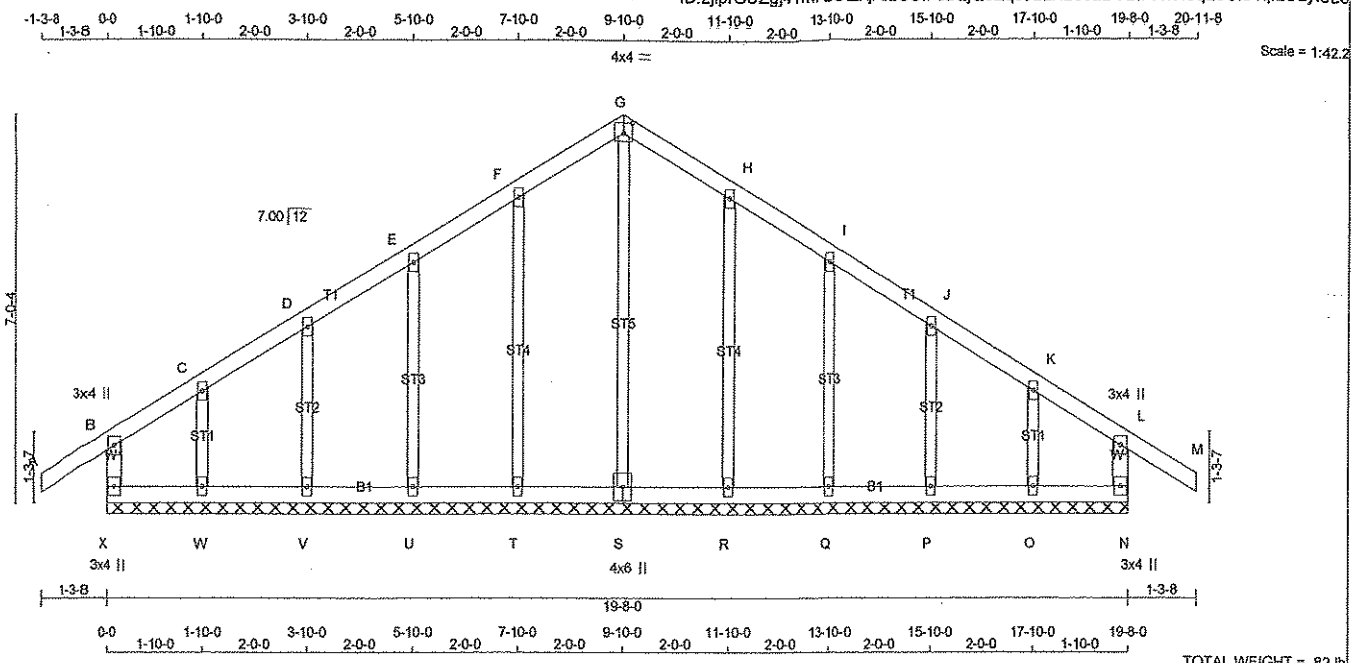

TOTAL WEIGHT = 82 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - G	2x4	DRY	No.2	SPF
G - M	2x4	DRY	No.2	SPF
X - B	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
X - S	2x4	DRY	No.2	SPF
S - N	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C, D, E, F, H, I, J, K						
C	TMW+w	MT20	2.0	4.0		
G	TTW-p	MT20	4.0	4.0	2.25	2.00
L	TMV+p	MT20	3.0	4.0		
N	BMV1+p	MT20	3.0	4.0		
O, P, Q, R, T, U, V, W						
O	BMW1+w	MT20	2.0	4.0		
S	BSW1+l	MT20	4.0	6.0		
X	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 29	-84.3	-84.3	0.11 (1)	10.00	S- G	-201 / 0	0.17 (1)
B- C	-6 / 2	-84.3	-84.3	0.08 (1)	10.00	T- F	-173 / 0	0.09 (1)
C- D	-5 / 22	-84.3	-84.3	0.05 (1)	10.00	U- E	-163 / 0	0.05 (1)
D- E	-5 / 22	-84.3	-84.3	0.05 (1)	10.00	V- D	-175 / 0	0.04 (1)
E- F	-4 / 27	-84.3	-84.3	0.05 (1)	10.00	W- C	-127 / 0	0.02 (1)
F- G	-4 / 28	-84.3	-84.3	0.05 (1)	10.00	R- H	-173 / 0	0.09 (1)
G- H	-4 / 28	-84.3	-84.3	0.05 (1)	10.00	Q- I	-163 / 0	0.05 (1)
H- I	-4 / 27	-84.3	-84.3	0.05 (1)	10.00	P- J	-175 / 0	0.04 (1)
I- J	-5 / 22	-84.3	-84.3	0.05 (1)	10.00	O- K	-127 / 0	0.02 (1)
J- K	-5 / 22	-84.3	-84.3	0.05 (1)	10.00			
K- L	-6 / 2	-84.3	-84.3	0.08 (1)	10.00			
L- M	0 / 29	-84.3	-84.3	0.11 (1)	10.00			
X- B	-204 / 0	0.0	0.0	0.03 (1)	7.81			
N- L	-204 / 0	0.0	0.0	0.03 (1)	7.81			
X- W	-11 / 5	-28.0	-28.0	0.02 (3)	6.25			
W- V	-16 / 4	-28.0	-28.0	0.02 (3)	6.25			
V- U	-20 / 4	-28.0	-28.0	0.02 (3)	6.25			
U- T	-23 / 4	-28.0	-28.0	0.02 (3)	6.25			
T- S	-25 / 4	-28.0	-28.0	0.02 (3)	6.25			
S- R	-25 / 4	-28.0	-28.0	0.02 (3)	6.25			
R- Q	-23 / 4	-28.0	-28.0	0.02 (3)	6.25			
Q- P	-20 / 4	-28.0	-28.0	0.02 (3)	6.25			
P- O	-16 / 4	-28.0	-28.0	0.02 (3)	6.25			
O- N	-11 / 5	-28.0	-28.0	0.02 (3)	6.25			

DWG NO. TAM44013-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11 (L-M:1) , BC=0.02 (N-O:3) , WB=0.17 (G-S:1) , SSI=0.08 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.40 (J) (INPUT = 0.90)
JSI METAL= 0.08 (S) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
282176	T18	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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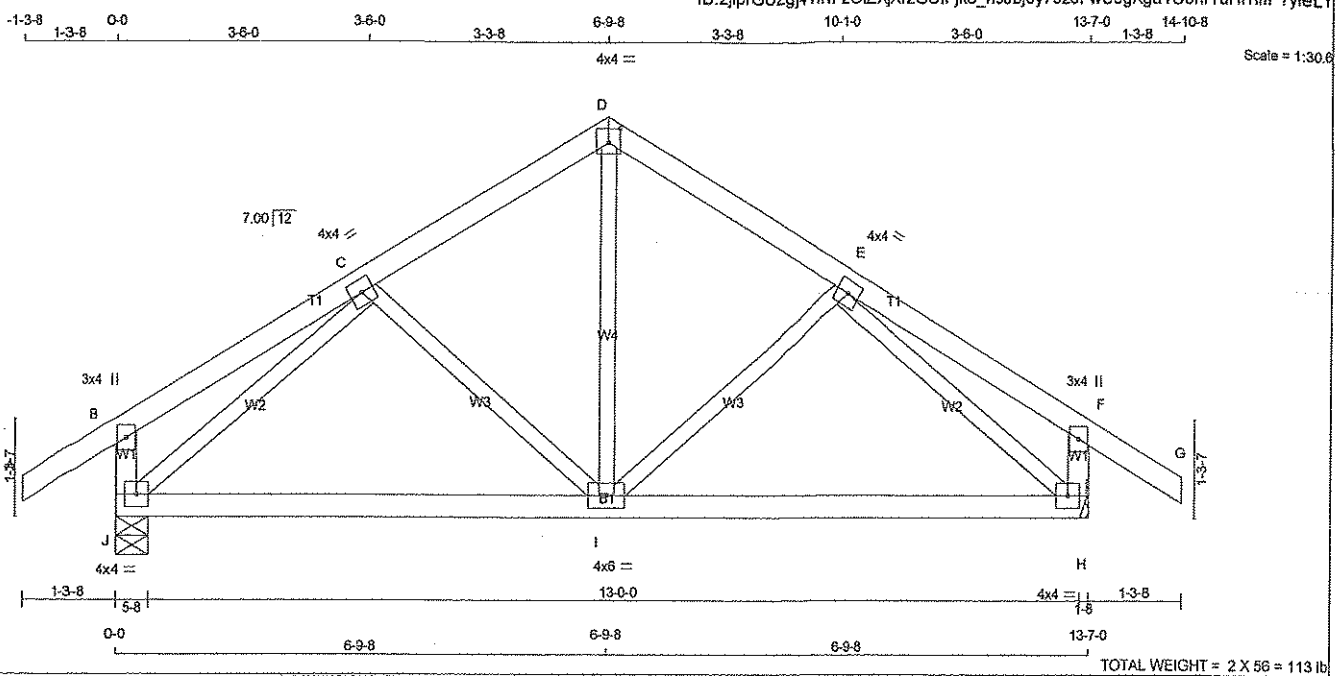

TOTAL WEIGHT = 2 X 56 = 113 lb

{M}{F}

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	877	0	877	0	0	5-8	5-8
H	877	0	877	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	704	417 / 0	143 / 0	0 / 0	0 / 0	144 / 0	0 / 0
H	704	417 / 0	143 / 0	0 / 0	0 / 0	144 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 29	-84.3	-84.3	0.11 (1)	10.00		I- D	0 / 425	0.10 (1)
B- C	0 / 16	-84.3	-84.3	0.15 (1)	10.00		I- E	-145 / 49	0.05 (1)
C- D	-659 / 0	-84.3	-84.3	0.12 (1)	6.25		C- I	-145 / 49	0.05 (1)
D- E	-659 / 0	-84.3	-84.3	0.12 (1)	6.25		J- C	-881 / 0	0.29 (1)
E- F	0 / 16	-84.3	-84.3	0.15 (1)	10.00		E- H	-881 / 0	0.29 (1)
F- G	0 / 29	-84.3	-84.3	0.11 (1)	10.00				
J- B	-227 / 0	0.0	0.0	0.02 (1)	7.81				
H- F	-227 / 0	0.0	0.0	0.02 (1)	7.81				
J- I	0 / 665	-28.0	-28.0	0.45 (2)	10.00				
I- H	0 / 665	-28.0	-28.0	0.45 (2)	10.00				

DWG NO. TAM 44034-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.45")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.45")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.15 (E-F:1) , BC=0.45 (H-I:2) , WB=0.29 (C-J:1) , SSI=0.16 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (J) (INPUT = 0.90)
JSI METAL= 0.31 (E) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
282177	G18	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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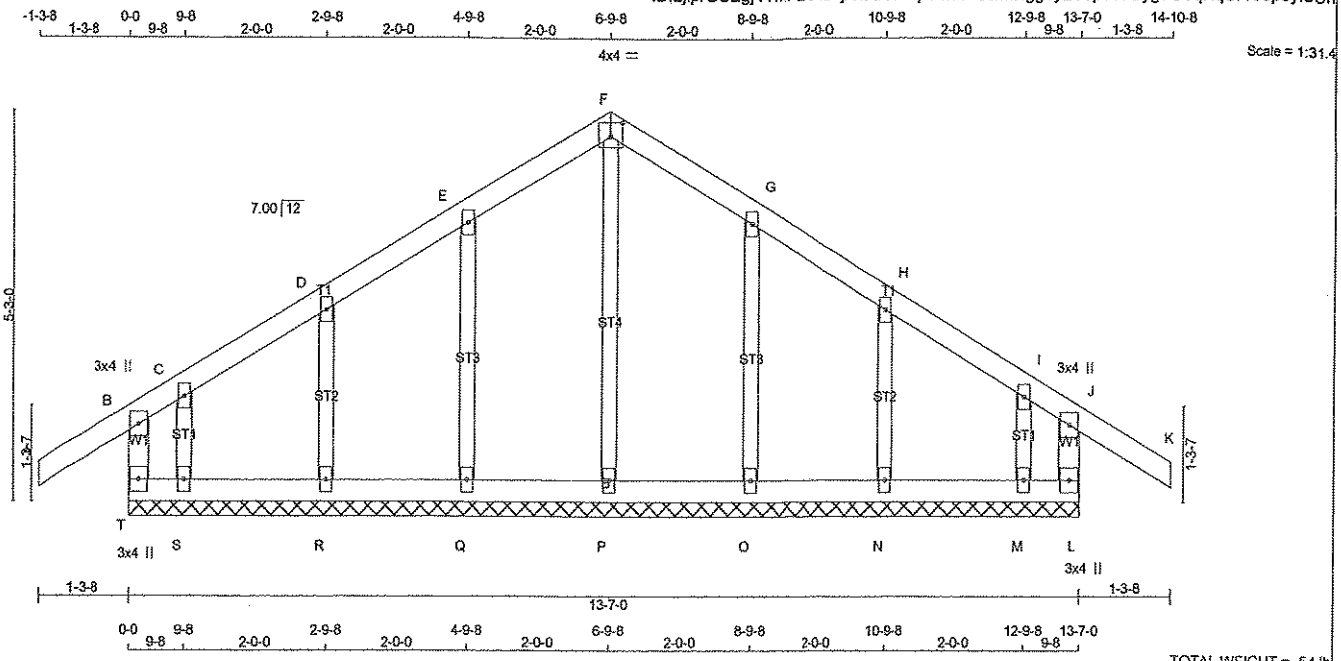

TOTAL WEIGHT = 54 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - F	2x4	DRY	No.2	SPF
F - K	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C, D, E, G, H, I						
C	TMW+w	MT20	2.0	4.0		
F	TTW-p	MT20	4.0	4.0	2.25	2.00
J	TMV+p	MT20	3.0	4.0		
L	BMV1+p	MT20	3.0	4.0		
M, N, O, P, Q, R, S						
M	BMW1+w	MT20	2.0	4.0		
T	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 29	-84.3	-84.3	0.11 (1)	10.00
B- C	-9 / 0	-84.3	-84.3	0.08 (1)	10.00
C- D	0 / 26	-84.3	-84.3	0.05 (1)	10.00
D- E	0 / 28	-84.3	-84.3	0.05 (1)	10.00
E- F	0 / 31	-84.3	-84.3	0.05 (1)	10.00
F- G	0 / 31	-84.3	-84.3	0.05 (1)	10.00
G- H	0 / 28	-84.3	-84.3	0.05 (1)	10.00
H- I	0 / 26	-84.3	-84.3	0.05 (1)	10.00
I- J	-9 / 0	-84.3	-84.3	0.08 (1)	10.00
J- K	0 / 29	-84.3	-84.3	0.11 (1)	10.00
T- B	-191 / 0	0.0	0.0	0.03 (1)	7.81
L- J	-191 / 0	0.0	0.0	0.03 (1)	7.81
T- S	-16 / 0	-28.0	-28.0	0.02 (3)	6.25
S- R	-19 / 0	-28.0	-28.0	0.02 (2)	6.25
R- Q	-24 / 0	-28.0	-28.0	0.02 (2)	6.25
Q- P	-27 / 0	-28.0	-28.0	0.02 (2)	6.25
P- O	-27 / 0	-28.0	-28.0	0.02 (2)	6.25
O- N	-24 / 0	-28.0	-28.0	0.02 (2)	6.25
N- M	-19 / 0	-28.0	-28.0	0.02 (2)	6.25
M- L	-16 / 0	-28.0	-28.0	0.02 (3)	6.25

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- F	-205 / 0	0.08 (1)
Q- E	-170 / 0	0.04 (1)
R- D	-173 / 0	0.03 (1)
S- C	-51 / 0	0.01 (1)
O- G	-170 / 0	0.04 (1)
N- H	-173 / 0	0.03 (1)
M- I	-51 / 0	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11 (J-K:1) , BC=0.02 (N-O:2) , WB=0.08 (F-P:1) , SSI=0.08 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1659

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.16 (F) (INPUT = 0.90)
JSI METAL= 0.05 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 4401B17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
282176	T18S	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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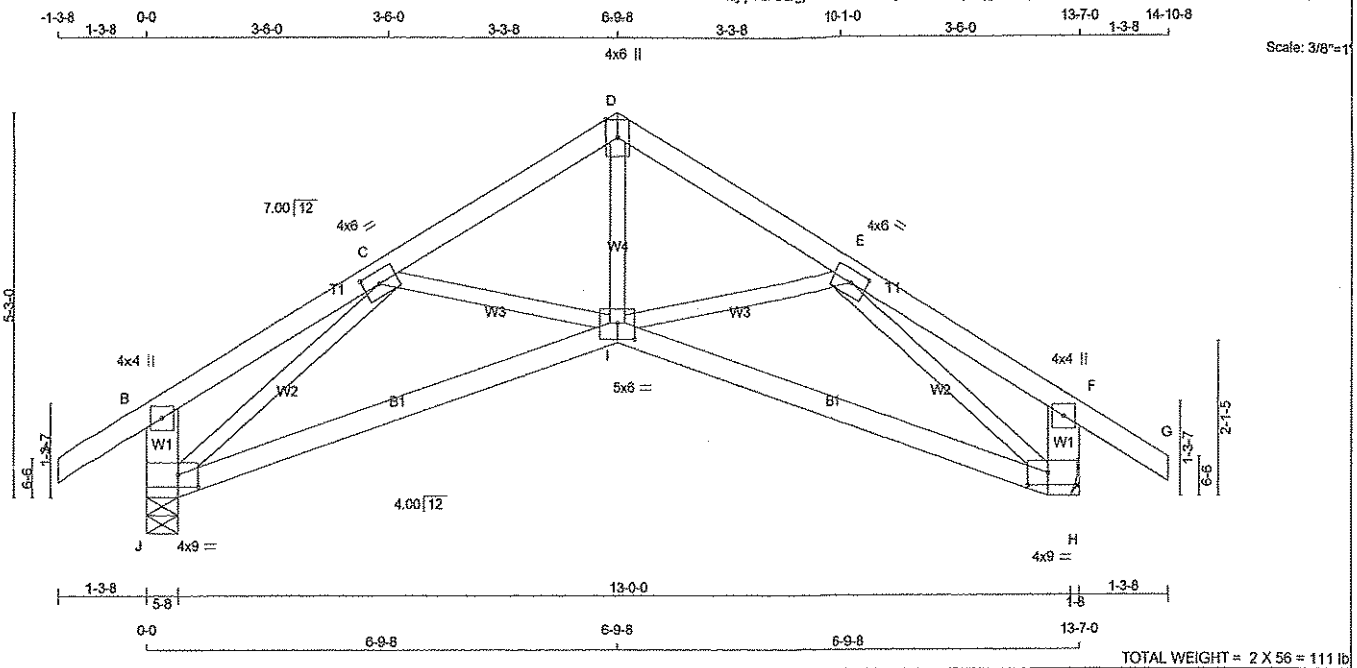

TOTAL WEIGHT = 2 X 56 = 111 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	4.0	6.0	2.00	2.75
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0	2.00	2.75
F	TMV+p	MT20	4.0	4.0		
H	BVMW1-p	MT20	4.0	9.0	2.00	3.50
I	BBWWW-p	MT20	5.0	6.0	2.75	3.00
J	BVMW1-p	MT20	4.0	9.0	2.00	3.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
J	877	0	877	0	0	5-8	5-8
H	877	0	877	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	704	417 / 0	143 / 0	0 / 0	0 / 0	144 / 0	0 / 0
H	704	417 / 0	143 / 0	0 / 0	0 / 0	144 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.96 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 29	-84.3	-84.3	0.11 (1)		10.00
B- C	0 / 13	-84.3	-84.3	0.13 (1)		10.00
C- D	-1078 / 0	-84.3	-84.3	0.11 (1)		5.96
D- E	-1078 / 0	-84.3	-84.3	0.11 (1)		5.96
E- F	0 / 13	-84.3	-84.3	0.13 (1)		10.00
F- G	0 / 29	-84.3	-84.3	0.11 (1)		10.00
J- B	-232 / 0	0.0	0.0	0.02 (1)		7.81
H- F	-232 / 0	0.0	0.0	0.02 (1)		7.81
J- I	0 / 1005	-28.0	-28.0	0.49 (2)		10.00
I- H	0 / 1005	-28.0	-28.0	0.49 (2)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- D	0 / 843	0.19 (1)
I- E	-35 / 112	0.03 (3)
C- I	-35 / 112	0.03 (3)
J- C	-1289 / 0	0.41 (1)
E- H	-1289 / 0	0.41 (1)

LICENSED PROFESSIONAL ENGINEER
83077
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 44005 17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.45")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.45")
CALCULATED VERT. DEFL.(TL) = L/ 614 (0.27")

CSI: TC=0.13 (B-C:1) , BC=0.49 (H-I:2) , WB=0.41 (C-J:1) , SSI=0.12 (H-I:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (J) (INPUT = 0.90)
JSI METAL= 0.31 (E) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
282176	T19	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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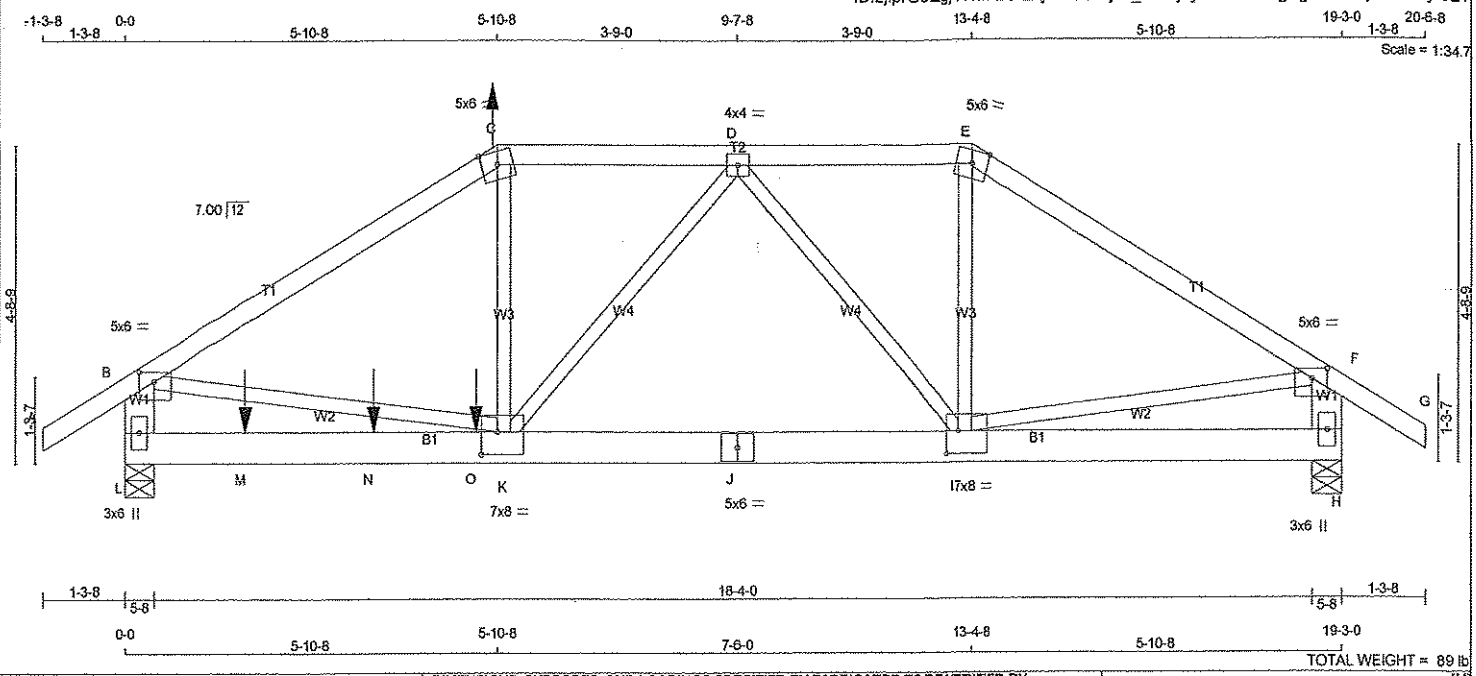

TOTAL WEIGHT = 89 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
L - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.75	2.75
C	TTW-m	MT20	5.0	6.0	Edge	
D	TMWW-t	MT20	4.0	4.0		
E	TTW-m	MT20	5.0	6.0	Edge	
F	TMVW-p	MT20	5.0	6.0	1.75	2.75
H	BMV1+p	MT20	3.0	6.0		
I	BMWWW-t	MT20	7.0	8.0	4.00	2.50
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	7.0	8.0	4.00	3.00
L	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1958.2 lbs FACTORED UP AT 5-10-8 ON TOP CHORD, AND 86.2 lbs FACTORED DOWN AT 1-11-4, AND 86.2 lbs FACTORED DOWN AT 3-11-4, AND 1246.3 lbs FACTORED DOWN AT 5-6-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	2045	0	2045	0	0	5-8	5-8
H	1516	0	1516	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1693	976 / -264	420 / 0	0 / 0	0 / 0	297 / 0	0 / 0
H	1233	730 / 0	276 / 0	0 / 0	0 / 0	227 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.52 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FR-TO	FACTORED VERT. LOAD (PLF)	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	FR-TO	MAX CSI (LC)
A- B	0 / 29	FROM TO	-84.3 -84.3	0.12 (1)	10.00		K- C	0 / 1285		0.32 (5)
B- C	-2380 / 609		-84.3 -84.3	0.82 (1)	3.52		K- D	-887 / 262		0.49 (5)
C- D	-2094 / 474		-84.3 -84.3	0.29 (5)	4.39		D- I	-725 / 381		0.40 (1)
D- E	-1481 / 0		-84.3 -84.3	0.23 (1)	5.10		I- E	-171 / 483		0.12 (2)
E- F	-1694 / 0		-84.3 -84.3	0.71 (1)	4.17		B- K	-531 / 2076		0.51 (1)
F- G	0 / 29		-84.3 -84.3	0.12 (1)	10.00		I- F	0 / 1478		0.37 (1)
L- B	-1866 / 22		0.0 0.0	0.13 (1)	7.29					
H- F	-1433 / 0		0.0 0.0	0.10 (1)	7.81					
L- M	0 / 0		-28.0 -28.0	0.32 (2)	10.00					
M- N	0 / 0		-28.0 -28.0	0.32 (2)	10.00					
N- O	0 / 0		-28.0 -28.0	0.32 (2)	10.00					
O- K	0 / 0		-28.0 -28.0	0.32 (2)	10.00					
K- J	0 / 1942		-28.0 -28.0	0.52 (1)	10.00					
J- I	0 / 1942		-28.0 -28.0	0.52 (1)	10.00					
I- H	0 / 0		-28.0 -28.0	0.16 (7)	10.00					

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE
C	5-10-8	174	—	195	BACK	VERT	DEAD
C	5-10-8	—	—	1958	BACK	VERT	TOTAL
M	1-11-4	-49	-86	—	BACK	VERT	TOTAL
N	3-11-4	-49	-86	—	BACK	VERT	TOTAL
O	5-6-8	-1246	-1246	—	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.64")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.64")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.82 (B-C:1) , BC=0.52 (I-K:1) , WB=0.51 (B-K:1) , SSI=0.89 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (F) (INPUT = 0.90)
JSI METAL= 0.48 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
83077
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 44026-17
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
282176	T20	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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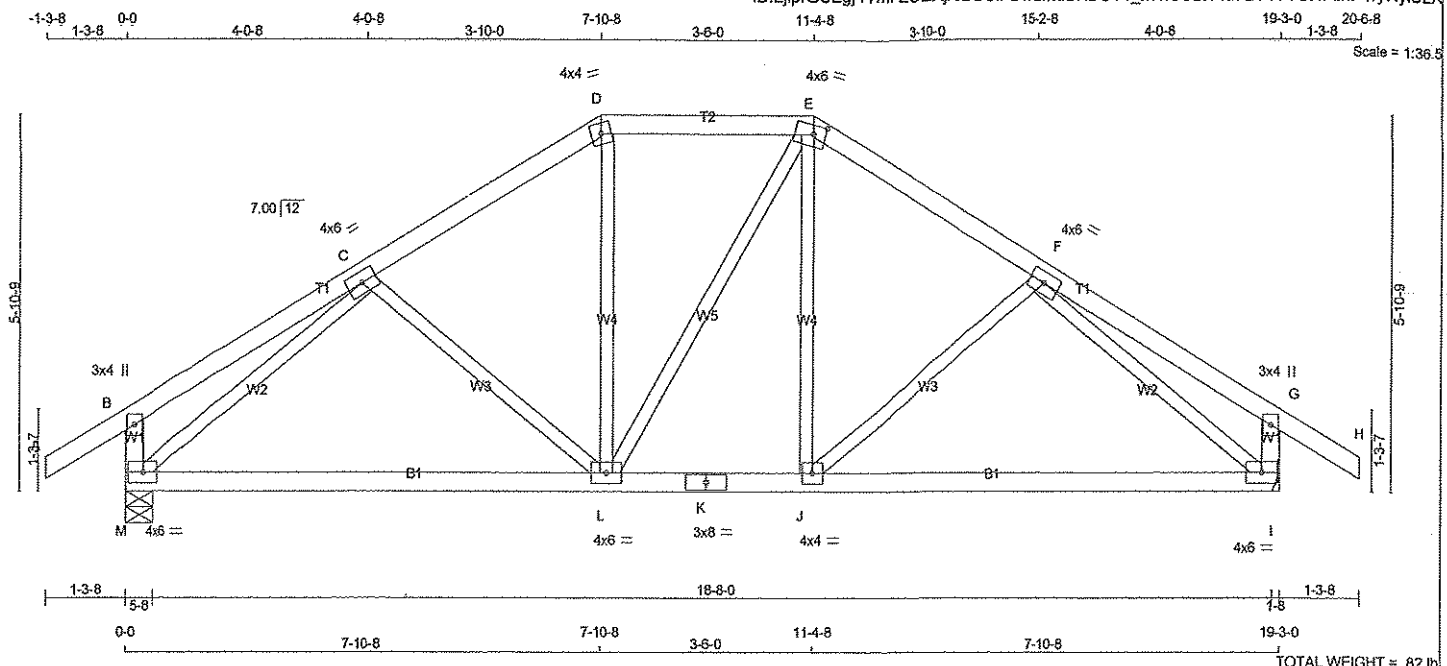

TOTAL WEIGHT = 82 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TTWW-m	MT20	4.0	6.0	1.75	2.25
F	TMWW-t	MT20	4.0	6.0		
G	TMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	8.0		
L	BMWWW-t	MT20	4.0	6.0		
M	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1195	0	1195	0	0	5-8	5-8
I	1195	0	1195	0	0	HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	965	562 / 0	202 / 0	0 / 0	0 / 0	201 / 0	0 / 0
I	965	562 / 0	202 / 0	0 / 0	0 / 0	201 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.86 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 29	-84.3	-84.3	1	0.11 (1)	10.00
B- C	0 / 19	-84.3	-84.3	1	0.20 (1)	10.00
C- D	-1088 / 0	-84.3	-84.3	1	0.17 (1)	5.88
D- E	-928 / 0	-84.3	-84.3	1	0.14 (1)	6.25
E- F	-1088 / 0	-84.3	-84.3	1	0.17 (1)	5.88
F- G	0 / 19	-84.3	-84.3	1	0.20 (1)	10.00
G- H	0 / 29	-84.3	-84.3	1	0.11 (1)	10.00
M- B	-244 / 0	0.0	0.0	1	0.02 (1)	7.81
I- G	-244 / 0	0.0	0.0	1	0.02 (1)	7.81
M- L	0 / 1033	-28.0	-28.0	1	0.48 (2)	10.00
L- K	0 / 927	-28.0	-28.0	1	0.46 (2)	10.00
K- J	0 / 927	-28.0	-28.0	1	0.46 (2)	10.00
J- I	0 / 1032	-28.0	-28.0	1	0.49 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- L	-142 / 69	0.06 (1)
L- D	0 / 301	0.07 (2)
L- E	0 / 1	0.00 (2)
J- E	0 / 300	0.07 (2)
J- F	-142 / 69	0.06 (1)
M- C	-1342 / 0	0.57 (1)
F- I	-1342 / 0	0.57 (1)

DWG NO. TAM 4007-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.64")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (0.64")
CALCULATED VERT. DEFL.(TL) = L/ 797 (0.29")

CSI: TC=0.20 (F-G;1) , BC=0.49 (I-J;2) , WB=0.57 (C-M;1) , SSI=0.18 (L-M;3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.71 (M) (INPUT = 0.90)
JSI METAL= 0.61 (K) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
282176	T21	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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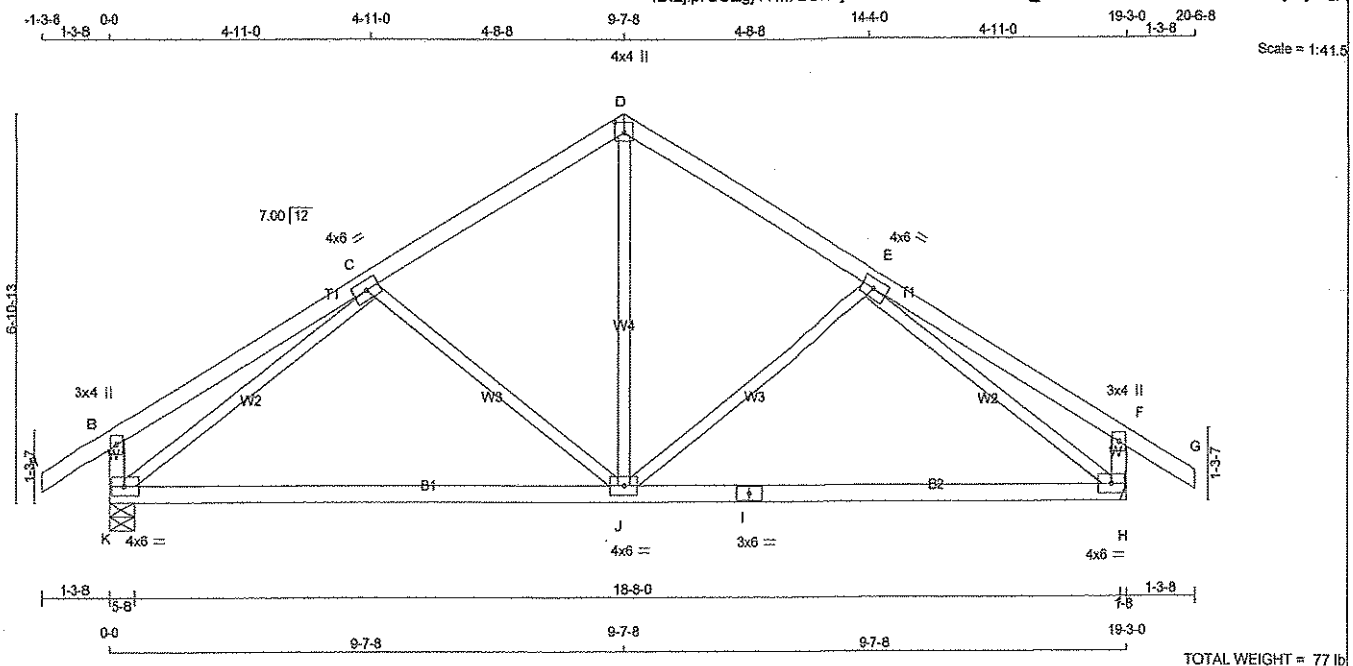

TOTAL WEIGHT = 77 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW+p	MT20	4.0	4.0	2.25	2.00
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
K	1195	0	1195	0	0	5-8	5-8
H	1195	0	1195	0	0	HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	965	562 / 0	202 / 0	0 / 0	0 / 0	201 / 0	0 / 0
H	965	562 / 0	202 / 0	0 / 0	0 / 0	201 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.96 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 29	-84.3	-84.3	0.11 (1)	10.00		J- D	0 / 666	0.15 (1)
B- C	0 / 24	-84.3	-84.3	0.31 (1)	10.00		J- E	-268 / 45	0.18 (1)
C- D	-993 / 0	-84.3	-84.3	0.25 (1)	5.96		C- J	-268 / 45	0.18 (1)
D- E	-993 / 0	-84.3	-84.3	0.25 (1)	5.96		K- C	-1332 / 0	0.65 (1)
E- F	0 / 24	-84.3	-84.3	0.31 (1)	10.00		E- H	-1332 / 0	0.65 (1)
F- G	0 / 29	-84.3	-84.3	0.11 (1)	10.00				
K- B	-271 / 0	0.0	0.0	0.03 (1)	7.81				
H- F	-271 / 0	0.0	0.0	0.03 (1)	7.81				
K- J	0 / 1048	-28.0	-28.0	0.87 (2)	10.00				
J- I	0 / 1048	-28.0	-28.0	0.87 (2)	10.00				
I- H	0 / 1048	-28.0	-28.0	0.87 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.64")
CALCULATED VERT. DEFL.(LL) = L/ 934 (0.25")
ALLOWABLE DEFL.(TL)= L/360 (0.64")
CALCULATED VERT. DEFL.(TL) = L/ 562 (0.41")

CSI: TC=0.31 (B-C:1) , BC=0.87 (J-K:2) , WB=0.86 (C-K:1) , SSI=0.23 (J-K:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1657	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

LSI GRIP= 0.84 (D) (INPUT = 0.90)
LSI METAL= 0.30 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 44008-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
282176	T22A	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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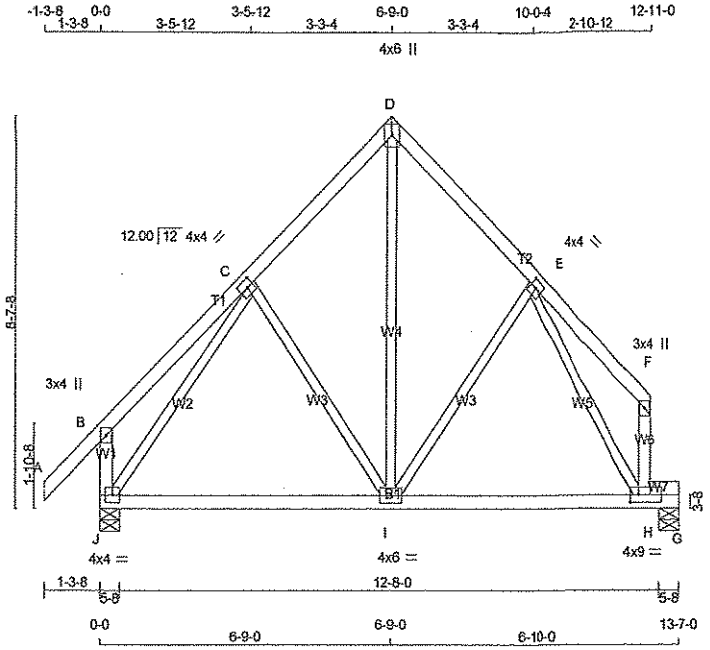

TOTAL WEIGHT = 4 X 68 = 273 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTW+p	MT20	4.0	6.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.75
F	TMV+p	MT20	3.0	4.0		
H	BMVWW-t	MT20	4.0	9.0	2.00	2.25
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	708	0	708	0	0	5-8	5-8
J	879	0	879	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	589	314 / 0	143 / 0	0 / 0	0 / 0	132 / 0	0 / 0
J	705	418 / 0	143 / 0	0 / 0	0 / 0	144 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 42	-84.3	-84.3	1	0.12 (1)	10.00
B- C	0 / 23	-84.3	-84.3	1	0.15 (1)	10.00
C- D	-506 / 0	-84.3	-84.3	1	0.15 (1)	6.25
D- E	-503 / 0	-84.3	-84.3	1	0.13 (1)	6.25
E- F	0 / 24	-84.3	-84.3	1	0.13 (1)	10.00
J- B	-225 / 0	0.0	0.0	1	0.03 (1)	7.81
H- F	-82 / 0	0.0	0.0	1	0.01 (1)	7.81
J- I	0 / 409	-28.0	-28.0	2	0.50 (2)	10.00
I- H	0 / 360	-28.0	-28.0	1	0.69 (1)	10.00
H- G	0 / 0	-28.0	-28.0	1	0.26 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- I	-116 / 37	0.07 (1)
I- D	0 / 455	0.10 (1)
I- E	-31 / 68	0.02 (1)
J- C	-723 / 0	0.43 (1)
E- H	-731 / 0	0.38 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.45")
CALCULATED VERT. DEFL.(LL) = L/ 802 (0.20")
ALLOWABLE DEFL.(TL)= L/360 (0.45")
CALCULATED VERT. DEFL.(TL) = L/ 475 (0.34")

CSI: TC=0.15 (C-D:1) , BC=0.69 (H-I:1) , WB=0.43 (C-J:1) , SSI=0.28 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI CSNF= 0.90 (E) (INPUT = 0.90)
JSI METAL= 0.33 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
8357
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 44094-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
282176	T23A	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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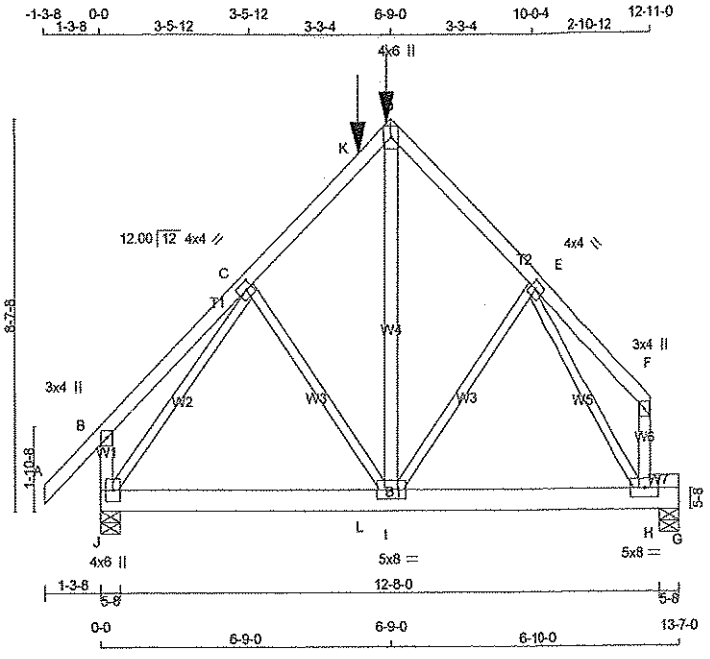

TOTAL WEIGHT = 2 X 79 = 157 lb

[M

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - G	2x6	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
I - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	SIDE(61.0)
D- F	1	12	SIDE(61.0)
J- B	1	12	TOP
H- F	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
J- G	2	12	SIDE(53.6)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
I- D	1	6	SIDE(9.9)

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTW+p	MT20	4.0	6.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.50
F	TMV+p	MT20	3.0	4.0		
H	BMVWW-t	MT20	5.0	8.0		
I	BMWWW-t	MT20	5.0	8.0		
J	BMVW1+p	MT20	4.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 511.8 lbs FACTORED DOWN AT 6-1-0, AND 505.5 lbs FACTORED DOWN AT 6-9-0 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1406	0	1406	0	0	5-8	5-8
J	1634	0	1634	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1187	606 / 0	306 / 0	0 / 0	0 / 0	275 / 0	0 / 0
J	1343	744 / 0	307 / 0	0 / 0	0 / 0	292 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 42	-84.3	-84.3	0.07 (1)	10.00		C- I	-72 / 91	0.02 (1)
B- C	0 / 39	-84.3	-84.3	0.14 (1)	10.00		I- D	0 / 823	0.07 (2)
C- K	-1156 / 0	-84.3	-84.3	0.25 (1)	6.25		I- E	0 / 191	0.02 (2)
K- D	-1156 / 0	-84.3	-84.3	0.25 (1)	6.25		J- C	-1590 / 0	0.45 (1)
D- E	-1245 / 0	-84.3	-84.3	0.07 (1)	6.25		E- H	-1533 / 0	0.38 (1)
E- F	0 / 22	-84.3	-84.3	0.07 (1)	10.00				
J- B	-202 / 0	0.0	0.0	0.01 (1)	7.81				
H- F	-85 / 0	0.0	0.0	0.01 (1)	7.81				
J- L	0 / 910	-58.6	-58.6	0.29 (2)	10.00				
L- I	0 / 910	-89.3	-89.3	0.29 (2)	10.00				
I- H	0 / 765	-58.6	-58.6	0.39 (1)	10.00				
H- G	0 / 0	-58.6	-58.6	0.21 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE
D	6-9-0	-506	-506	---	FRONT	VERT	TOTAL
K	6-1-0	-512	-512	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 6-9-0
RIGHT SETBACK = 6-10-0
END SETBACK = 6-10-0
END WALL WIDTH = 5-8
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.45")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.45")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.25 (C-D:1) , BC=0.39 (H-I:1) , WB=0.45 (C-J:1) , SSI=0.28 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (C) (INPUT = 0.90)
JSI METAL= 0.30 (C) (INPUT = 1.00)

DWG NO. TAM 44012-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
282177	T24	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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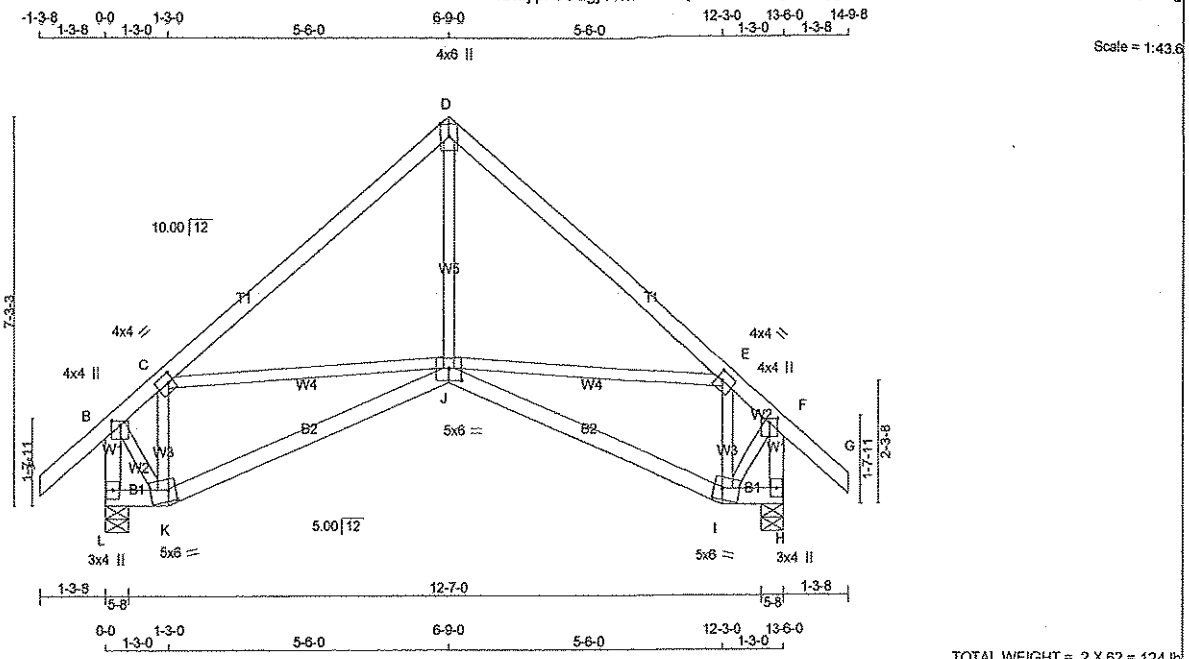

TOTAL WEIGHT = 2 X 62 = 124 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	4.0	2.00	1.25
F	TMVW+p	MT20	4.0	4.0	1.00	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BBWW-m	MT20	5.0	6.0	2.50	1.75
J	BBWWW-p	MT20	5.0	6.0	2.75	3.00
K	BBWW-m	MT20	5.0	6.0	2.50	1.75
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	874	0	874	0	0	5-8	5-8
H	874	0	874	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	701	416 / 0	142 / 0	0 / 0	0 / 0	143 / 0	0 / 0
H	701	416 / 0	142 / 0	0 / 0	0 / 0	143 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 37	-84.3	-84.3	0.12 (1)	10.00	J- D	0 / 513	0.12 (1)	
B- C	-490 / 0	-84.3	-84.3	0.27 (1)	6.25	J- E	0 / 109	0.02 (2)	
C- D	-726 / 0	-84.3	-84.3	0.32 (1)	6.25	I- E	-555 / 0	0.09 (1)	
D- E	-726 / 0	-84.3	-84.3	0.32 (1)	6.25	C- J	0 / 109	0.02 (2)	
E- F	-490 / 0	-84.3	-84.3	0.27 (1)	6.25	K- C	-555 / 0	0.09 (1)	
F- G	0 / 37	-84.3	-84.3	0.12 (1)	10.00	B- K	0 / 653	0.15 (1)	
L- B	-857 / 0	0.0	0.0	0.09 (1)	7.81	I- F	0 / 653	0.15 (1)	
H- F	-857 / 0	0.0	0.0	0.09 (1)	7.81				
L- K	0 / 0	-28.0	-28.0	0.01 (2)	10.00				
K- J	0 / 463	-28.0	-28.0	0.30 (2)	10.00				
J- I	0 / 463	-28.0	-28.0	0.30 (2)	10.00				
I- H	0 / 0	-28.0	-28.0	0.01 (2)	10.00				

DWG NO. TAM 44014-17
STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.45")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.45")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.32 (D-E:1) , BC=0.30 (J-K:2) , WB=0.15 (B-K:1) , SSI=0.17 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (K) (INPUT = 0.90)
JSI METAL= 0.16 (F) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43994	DRWG NO.
282177	T25	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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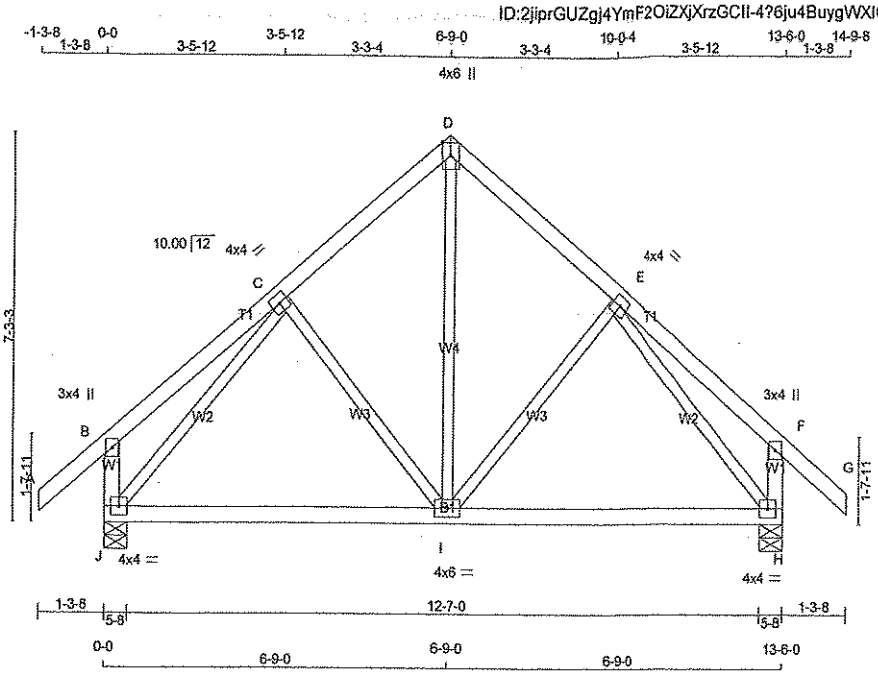


TOTAL WEIGHT = 2 X 64 = 128 lb

[MiTek]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	874	0	874	0	0	5-8	5-8
H	874	0	874	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	701	416 / 0	142 / 0	0 / 0	0 / 0	143 / 0	0 / 0
H	701	416 / 0	142 / 0	0 / 0	0 / 0	143 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 37	-84.3	-84.3	0.12 (1)	10.00
B- C	0 / 21	-84.3	-84.3	0.15 (1)	10.00
C- D	-529 / 0	-84.3	-84.3	0.12 (1)	6.25
D- E	-529 / 0	-84.3	-84.3	0.12 (1)	6.25
E- F	0 / 21	-84.3	-84.3	0.15 (1)	10.00
F- G	0 / 37	-84.3	-84.3	0.12 (1)	10.00
J- B	-225 / 0	0.0	0.0	0.02 (1)	7.81
H- F	-225 / 0	0.0	0.0	0.02 (1)	7.81
J- I	0 / 472	-28.0	-28.0	0.42 (2)	10.00
I- H	0 / 472	-28.0	-28.0	0.42 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- D	0 / 428	0.10 (1)
I- E	-126 / 38	0.06 (1)
C- I	-126 / 38	0.06 (1)
J- C	-745 / 0	0.34 (1)
E- H	-745 / 0	0.34 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.45")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.45")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.15 (E-F:1) , BC=0.42 (I-J:2) , WB=0.34 (E-H:1) , SSI=0.16 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

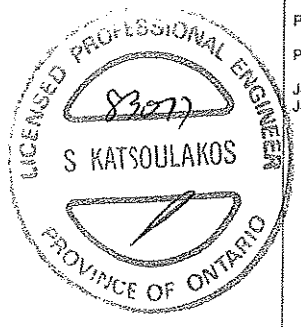
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (C) (INPUT = 0.90)
JSI METAL= 0.27 (E) (INPUT = 1.00)



DWG NO. TAM 44015-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
282177	T25A	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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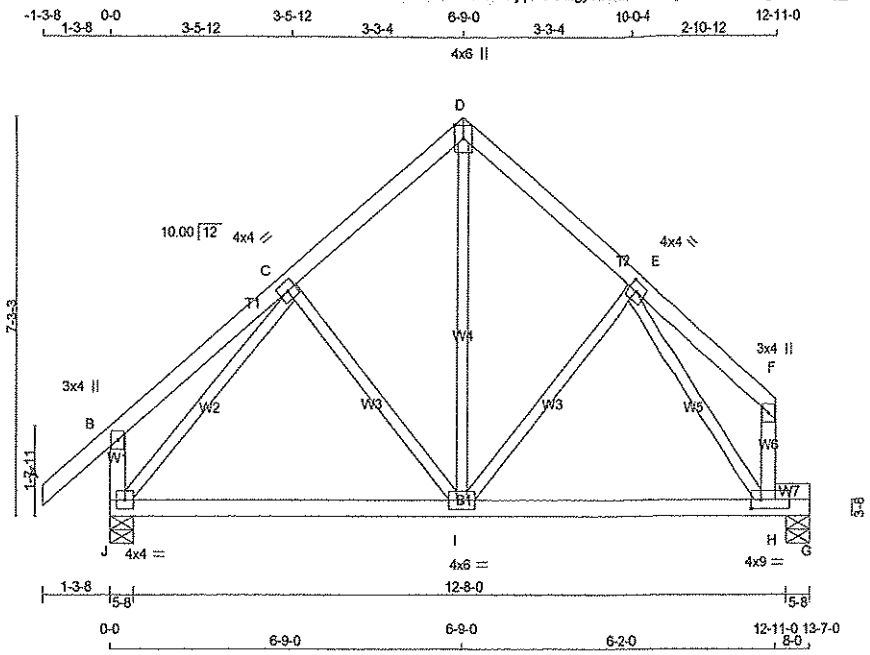

TOTAL WEIGHT = 4 X 62 = 250 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	4.0	2.00	1.75
F	TMV+p	MT20	3.0	4.0		
H	BMVWW-t	MT20	4.0	9.0	2.00	2.25
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	708	0	708	0	0	5-8	5-8
J	878	0	878	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	589	314 / 0	143 / 0	0 / 0	0 / 0	132 / 0	0 / 0
J	704	418 / 0	143 / 0	0 / 0	0 / 0	144 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 37	-84.3	-84.3	0.12 (1)	10.00
B- C	0 / 21	-84.3	-84.3	0.15 (1)	10.00
C- D	-552 / 0	-84.3	-84.3	0.15 (1)	6.25
D- E	-549 / 0	-84.3	-84.3	0.13 (1)	6.25
E- F	0 / 21	-84.3	-84.3	0.13 (1)	10.00
J- B	-225 / 0	0.0	0.0	0.02 (1)	7.81
H- F	-83 / 0	0.0	0.0	0.01 (1)	7.81
J- I	0 / 486	-28.0	-28.0	0.51 (2)	10.00
I- H	0 / 429	-28.0	-28.0	0.70 (1)	10.00
H- G	0 / 0	-28.0	-28.0	0.25 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- D	0 / 452	0.10 (1)
I- E	-32 / 73	0.02 (3)
C- I	-122 / 40	0.06 (1)
J- C	-768 / 0	0.35 (1)
E- H	-766 / 0	0.30 (1)

DWG NO. TAM 44016-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F, RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.45")
CALCULATED VERT. DEFL.(LL) = L/ 801 (0.20")
ALLOWABLE DEFL.(TL)= L/360 (0.45")
CALCULATED VERT. DEFL.(TL) = L/ 475 (0.34")

CSI: TC=0.15 (C-D:1) , BC=0.70 (H-I:1) , WB=0.35 (C-J:1) , SSI=0.28 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (C) (INPUT = 0.90)
JSI METAL= 0.32 (H) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
282177	T26	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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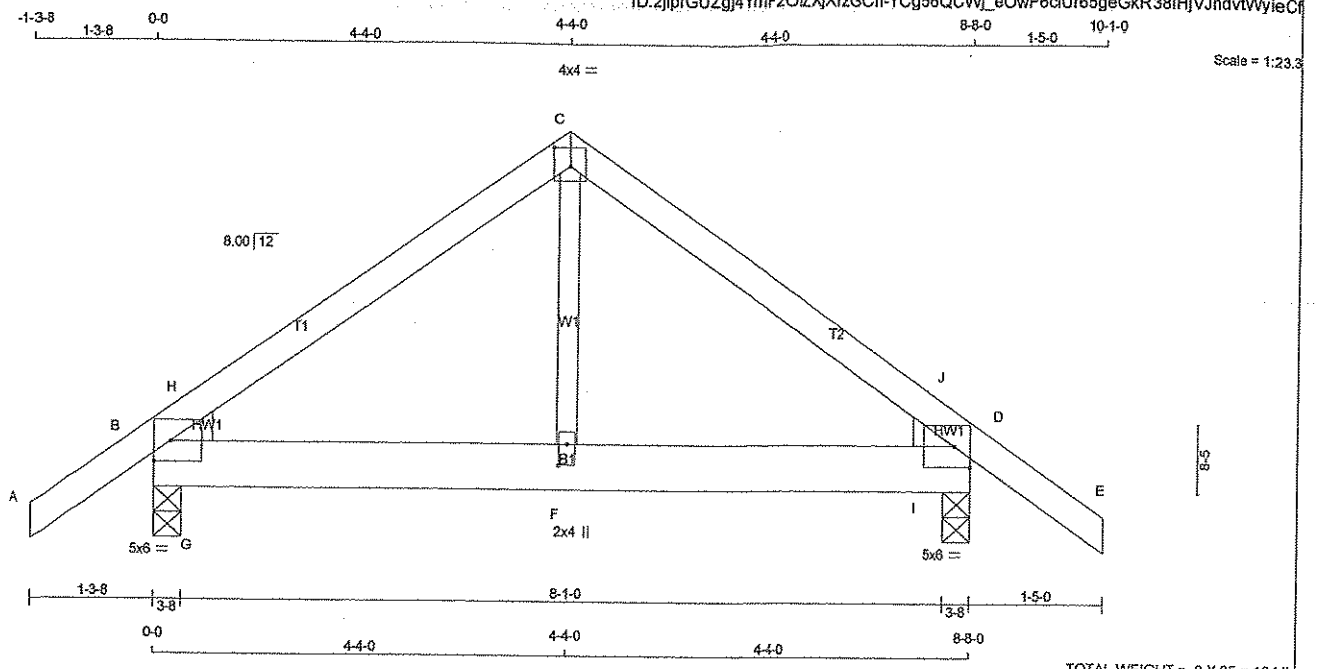

TOTAL WEIGHT = 3 X 35 = 104 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-I	MT20	5.0	6.0		Edge
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMBH1-I	MT20	5.0	6.0		Edge
F	BMW+w	MT20	2.0	4.0	2.50	1.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
B	602	0	602	0	0	3-8	3-8	2x4 L
D	613	0	613	0	0	3-8	3-8	2x4 R

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	478	292 / 0	91 / 0	0 / 0	0 / 0	95 / 0	0 / 0
D	485	298 / 0	91 / 0	0 / 0	0 / 0	96 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 24	-84.3	-84.3	0.11 (1)	10.00	F- C	0 / 256	0.06 (2)	
B- H	-400 / 0	-84.3	-84.3	0.12 (1)	6.25	G- H	-228 / 95	0.00 (1)	
H- C	-452 / 0	-84.3	-84.3	0.14 (1)	6.25	I- J	-228 / 95	0.00 (1)	
C- J	-452 / 0	-84.3	-84.3	0.14 (1)	6.25				
J- D	-400 / 0	-84.3	-84.3	0.12 (1)	6.25				
D- E	0 / 27	-84.3	-84.3	0.13 (1)	10.00				
B- G	0 / 363	-28.0	-28.0	0.12 (1)	10.00				
G- F	0 / 363	-28.0	-28.0	0.12 (1)	10.00				
F- I	0 / 363	-28.0	-28.0	0.12 (1)	10.00				
I- D	0 / 363	-28.0	-28.0	0.12 (1)	10.00				

LICENCED PROFESSIONAL ENGINEER
83671
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 4401717
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.29")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.29")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.14 (C-H:1) , BC=0.12 (F-G:1) , WB=0.06 (C-F:2) , SSI=0.15 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.43 (C) (INPUT = 0.90)
JSI METAL= 0.11 (D) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
282177	G26	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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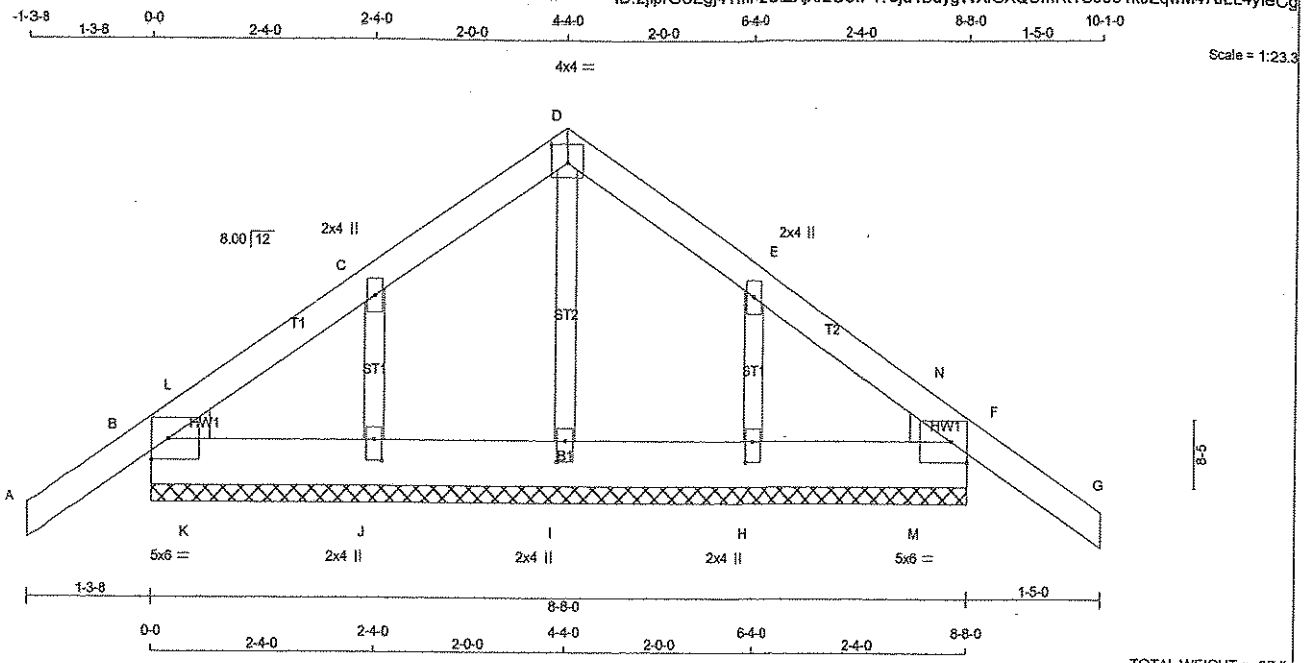

TOTAL WEIGHT = 37 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
B - F	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-I	MT20	5.0	6.0		Edge
C	TMW+w	MT20	2.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMW+w	MT20	2.0	4.0		
F	TMBH1-I	MT20	5.0	6.0		Edge
H, I, J						
H	BMW1+w	MT20	2.0	4.0	2.50	1.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

HEEL
WEDGE
2x4 L
2x4 R
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI	LC	MAX. UNBRAC LENGTH
A- B	0 / 24	-84.3	-84.3	0.11	(1)	10.00
B- L	-65 / 0	-84.3	-84.3	0.01	(1)	6.25
L- C	-47 / 0	-84.3	-84.3	0.05	(1)	6.25
C- D	-58 / 0	-84.3	-84.3	0.05	(1)	6.25
D- E	-56 / 0	-84.3	-84.3	0.05	(1)	6.25
E- N	-47 / 0	-84.3	-84.3	0.05	(1)	6.25
N- F	-65 / 0	-84.3	-84.3	0.01	(1)	6.25
F- G	0 / 27	-84.3	-84.3	0.13	(1)	10.00
B- K	0 / 47	-28.0	-28.0	0.03	(1)	10.00
K- J	0 / 47	-28.0	-28.0	0.03	(1)	10.00
J- I	0 / 38	-28.0	-28.0	0.02	(2)	10.00
I- H	0 / 38	-28.0	-28.0	0.02	(2)	10.00
H- M	0 / 47	-28.0	-28.0	0.03	(1)	10.00
M- F	0 / 47	-28.0	-28.0	0.03	(1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- D	-88 / 2	0.02 (1)
J- C	-193 / 0	0.03 (1)
H- E	-193 / 0	0.03 (1)
K- L	-62 / 9	0.00 (1)
M- N	-62 / 9	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.13 (F-G:1) , BC=0.03 (B-K:1) , WB=0.03 (C-J:1) , SSI=0.09 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (F) (INPUT = 0.90)
JSI METAL= 0.05 (C) (INPUT = 1.00)

DWG NO. TAM 44017
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
282178	T27	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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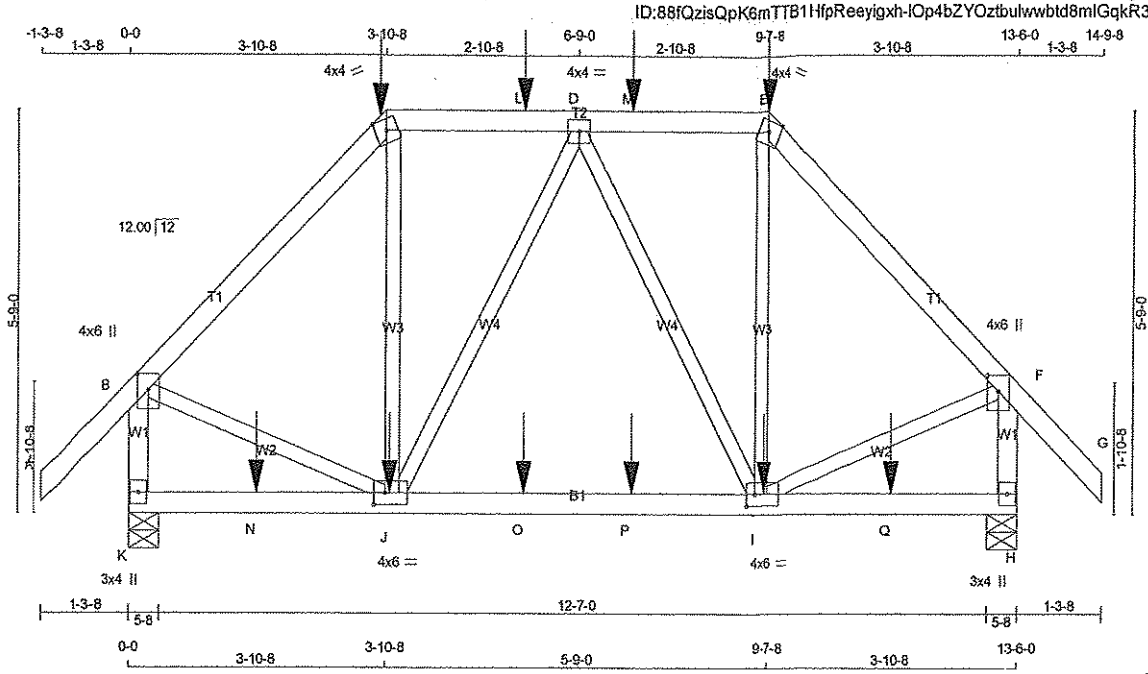


TOTAL WEIGHT = 67 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C	TTW-m	MT20	4.0	4.0	Edge	
D	TMWW-t	MT20	4.0	4.0		
E	TTW-m	MT20	4.0	4.0	Edge	
F	TMVW+p	MT20	4.0	6.0	2.75	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMWWW-t	MT20	4.0	6.0	2.00	1.50
J	BMWWW-t	MT20	4.0	6.0	2.00	2.00
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 225.2 lbs FACTORED DOWN AT 3-10-8, 38.3 lbs FACTORED DOWN AT 5-11-4, AND 38.3 lbs FACTORED DOWN AT 7-6-12, AND 225.2 lbs FACTORED DOWN AT 9-7-8 ON TOP CHORD, AND 42.6 lbs FACTORED DOWN AT 1-11-4, 35.0 lbs FACTORED DOWN AT 3-11-4, 35.0 lbs FACTORED DOWN AT 5-11-4, 35.0 lbs FACTORED DOWN AT 7-6-12, AND 35.0 lbs FACTORED DOWN AT 9-6-12, AND 42.6 lbs FACTORED DOWN AT 11-6-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	1203	0	1203	0	0	5-8	5-8
H	1203	0	1203	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	962	575 / 0	190 / 0	0 / 0	0 / 0	197 / 0	0 / 0
H	962	575 / 0	190 / 0	0 / 0	0 / 0	197 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.14 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

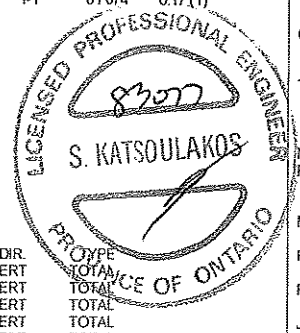
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 42	-84.3	-84.3	0.13 (1)	10.00	
B- C	-894 / 0	-84.3	-84.3	0.26 (1)	6.14	
C- L	-632 / 0	-84.3	-84.3	0.15 (1)	6.25	
L- D	-632 / 0	-84.3	-84.3	0.15 (1)	6.25	
D- M	-632 / 0	-84.3	-84.3	0.15 (1)	6.25	
M- E	-632 / 0	-84.3	-84.3	0.15 (1)	6.25	
E- F	-894 / 0	-84.3	-84.3	0.26 (1)	6.14	
F- G	0 / 42	-84.3	-84.3	0.13 (1)	10.00	
K- B	-1156 / 0	0.0	0.0	0.14 (1)	7.37	
H- F	-1156 / 0	0.0	0.0	0.14 (1)	7.37	
K- N	0 / 0	-28.0	-28.0	0.23 (3)	10.00	
N- J	0 / 0	-28.0	-28.0	0.23 (3)	10.00	
J- O	0 / 721	-28.0	-28.0	0.31 (2)	10.00	
O- P	0 / 721	-28.0	-28.0	0.31 (2)	10.00	
P- I	0 / 721	-28.0	-28.0	0.31 (2)	10.00	
I- Q	0 / 0	-28.0	-28.0	0.23 (3)	10.00	
Q- H	0 / 0	-28.0	-28.0	0.23 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- C	0 / 257	0.06 (3)
J- D	-197 / 0	0.13 (1)
D- I	-197 / 0	0.13 (1)
I- E	0 / 257	0.06 (3)
B- J	0 / 674	0.17 (1)
I- F	0 / 674	0.17 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	3-10-8	-225	-225	---	FRONT	VERT	TOTAL
E	9-7-8	-225	-225	---	FRONT	VERT	TOTAL
I	9-6-12	-20	-35	---	FRONT	VERT	TOTAL
J	3-11-4	-20	-35	---	FRONT	VERT	TOTAL
L	5-11-4	-38	-38	---	FRONT	VERT	TOTAL
M	7-6-12	-38	-38	---	FRONT	VERT	TOTAL
N	1-11-4	-24	-43	---	FRONT	VERT	TOTAL
O	5-11-4	-20	-35	---	FRONT	VERT	TOTAL
P	7-6-12	-20	-35	---	FRONT	VERT	TOTAL
Q	11-6-12	-24	-43	---	FRONT	VERT	TOTAL



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.45")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.45")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.26 (B-C:1) , BC=0.31 (I-J:2) , WB=0.17 (F-I:1) , SSI=0.16 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (B) (INPUT = 0.90)
JSI METAL= 0.22 (B) (INPUT = 1.00)

DWG NO. TAM 44020 17
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 43954	DRWG NO.
282178	T28	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Wed Aug 30 15:18:59 2017 Page 1

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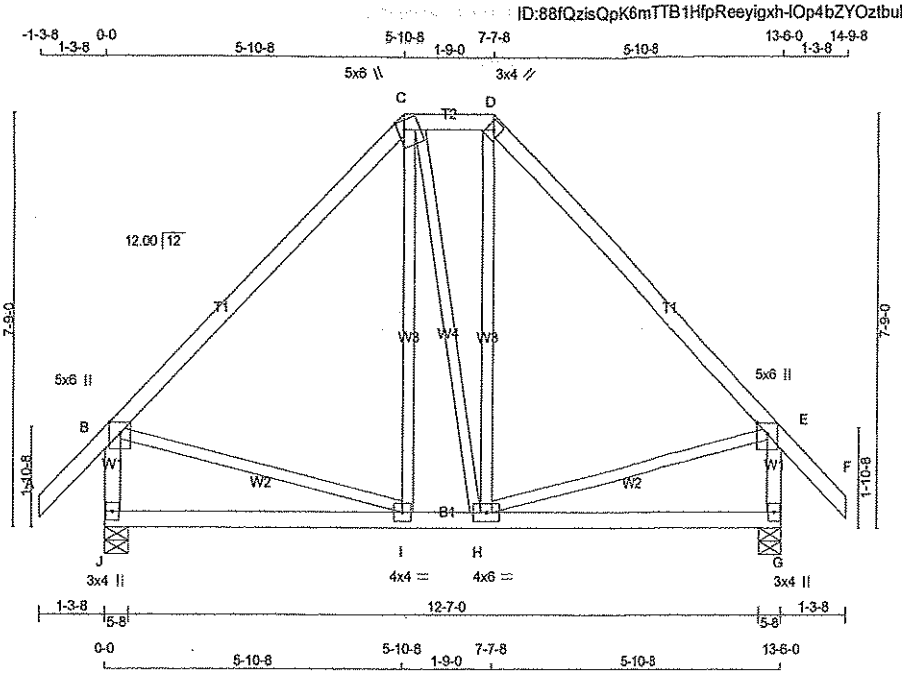


TOTAL WEIGHT = 72 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TTW+h	MT20	3.0	4.0		
E	TMVW+p	MT20	5.0	6.0	Edge	
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	875	0	875	0	0	5-8	5-8
G	875	0	875	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	702	417 / 0	142 / 0	0 / 0	0 / 0	143 / 0	0 / 0
G	702	417 / 0	142 / 0	0 / 0	0 / 0	143 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 42	-84.3	-84.3	0.12 (1)	10.00	I- C	0 / 150	0.03 (3)
B- C	-497 / 0	-84.3	-84.3	0.38 (1)	6.25	C- H	0 / 13	0.00 (3)
C- D	-352 / 0	-84.3	-84.3	0.03 (1)	6.25	H- D	0 / 162	0.04 (3)
D- E	-497 / 0	-84.3	-84.3	0.38 (1)	6.25	B- I	0 / 361	0.08 (1)
E- F	0 / 42	-84.3	-84.3	0.12 (1)	10.00	H- E	0 / 362	0.08 (1)
J- B	-806 / 0	0.0	0.0	0.09 (1)	7.81			
G- E	-805 / 0	0.0	0.0	0.09 (1)	7.81			
J- I	0 / 0	-28.0	-28.0	0.21 (3)	10.00			
I- H	0 / 351	-28.0	-28.0	0.27 (2)	10.00			
H- G	0 / 0	-28.0	-28.0	0.23 (3)	10.00			

DWG NO. TAM 4402-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.45")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.45")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.38 (D-E:1) , BC=0.27 (H-I:2) , WB=0.08 (E-H:1) , SSI=0.14 (G-H:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (D) (INPUT = 0.90)
JSI METAL= 0.13 (I) (INPUT = 1.00)

TECHNICAL BULLETIN

LUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

MATERIAL: 18 gauge

FINISH: G90 galvanized

DESIGN:

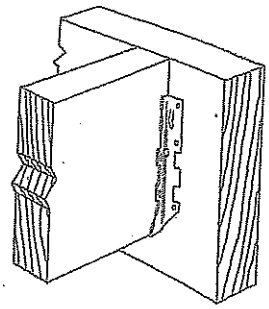
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

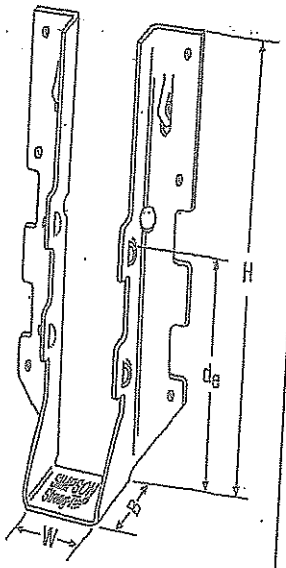
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

- These hangers cannot be modified.



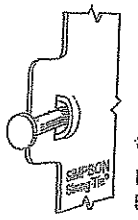
Typical LUS Installation



LUS28

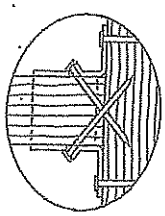
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LUS24	18	1⅜	3⅛	1¾	1⅝	4-10d	2-10d	710	1630	645	1155
LUS24-2	18	3⅛	3⅛	2	1⅜	4-16d	2-16d	895	2020	590	1435
LUS26	18	1⅜	4¾	1¾	3⅝	4-10d	4-10d	1420	2170	1290	1630
LUS26-2	18	3⅛	4⅞	2	4	4-16d	4-16d	1720	2595	1545	1920
LUS26-3	18	4⅝	4⅞	2	3¼	4-16d	4-16d	1720	2595	1545	2340
LUS28	18	1⅜	6⅝	1¾	3¾	6-10d	4-10d	1420	2520	1290	1790
LUS28-2	18	3⅛	7	2	4	6-16d	4-16d	1720	3325	1545	2575
LUS28-3	18	4⅝	6¼	2	3¼	6-16d	4-16d	1720	3325	1545	2375
LUS210	18	1⅜	7⅝	1¾	3⅝	8-10d	4-10d	1420	2785	1290	2210
LUS210-2	18	3⅛	9	2	6	8-16d	6-16d	2580	4500	2320	3195
LUS210-3	18	4⅝	8⅝	2	5¼	8-16d	6-16d	2590	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.

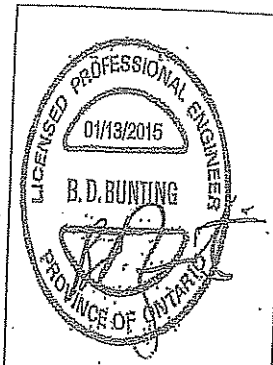


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2015. This information is updated periodically and should not be relied upon after December 31, 2016. Contact Simpson Strong-Tie for current information and limited warranty or see www.strongtie.com.

T-SPECLUS15 1/15 exp. 12/16

800.999.5099
www.strongtie.com

TECHNICAL BULLETIN

HUS/LJS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: See table

FINISH: G90 galvanized

DESIGN:

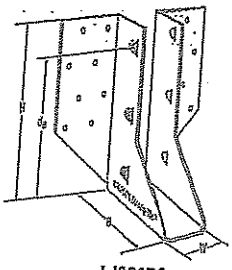
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15% No further increase is permitted
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

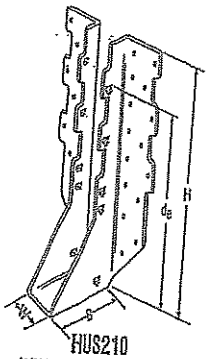
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

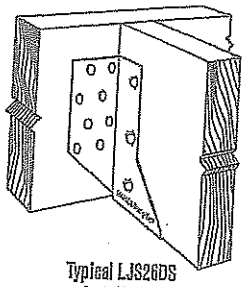
- See current catalogue for options



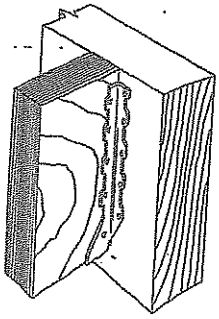
LJS26DS



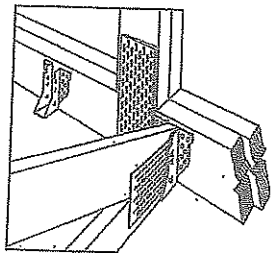
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS Installation



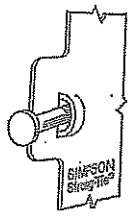
Typical HUS Installation



Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

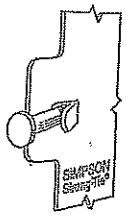
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
								D.Fir-L.		S-P-F	
		W	H	B	d_B[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LJS26DS	18	1⅜	5	3½	4⅝	16-16d	6-16d	2055	4265	1460	4115
HUS26	16	1⅝	5⅜	3	3¹⁵⁄₁₆	14-16d	6-16d	2705	4940	2065	3875
HUS28	16	1⅝	7³⁄₃₂	3	6³⁄₃₂	22-16d	8-16d	3605	5365	2675	4345
HUS210	16	1⅝	9³⁄₃₂	3	7³¹⁄₃₂	30-16d	10-16d	4505	5795	4010	4740
HUS1.81/10	16	1¹³⁄₁₆	9	3	8	30-16d	10-16d	4505	6450	4010	5200

1. d_B is the distance from the seat of the hanger to the highest joist nail.

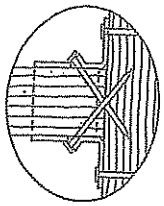


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

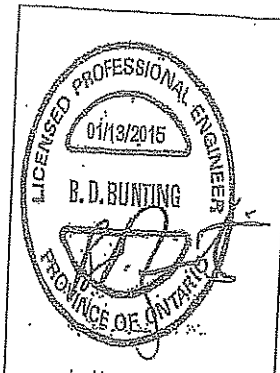
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



800 999-5099
www.strongtie.com

TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

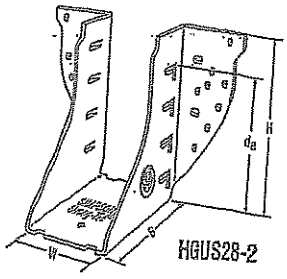
All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: 12 gauge

FINISH: G90 galvanized

DESIGN:

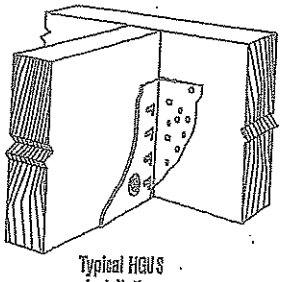
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.



HGUS28-2

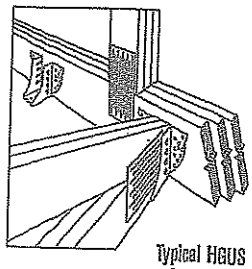
INSTALLATION:

- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications


Typical HGUS Installation

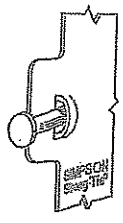
OPTIONS:

- See current catalogue for options

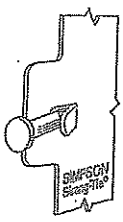

Typical HGUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)

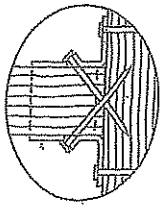
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
								D.Fir-L		S-P-F	
		W	H	B	d_g[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
HGUS26	12	1⅜	5⅜	5	4⅝	20-16d	8-16d	2685	6525	2685	5700
HGUS26-2	12	3⅜	5⅜	4	4⅛	20-16d	8-16d	4385	8950	3100	6955
HGUS26-3	12	4⅜	5⅜	4	4⅛	20-16d	8-16d	4385	8950	3100	6955
HGUS26-4	12	6⅜	5⅜	4	4⅛	20-16d	8-16d	4385	8950	3100	6955
HGUS28	12	1⅜	7⅛	5	6⅛	36-16d	12-16d	3310	7675	3100	6900
HGUS28-2	12	3⅜	7⅛	4	6⅛	36-16d	12-16d	6070	12980	4310	9215
HGUS28-3	12	4⅜	7¼	4	6⅛	36-16d	12-16d	6070	12980	4310	9215
HGUS28-4	12	6⅜	7⅛	4	6⅛	36-16d	12-16d	6070	12980	4310	9215
HGU210-2	12	3⅜	9⅛	4	8⅛	46-16d	16-16d	6840	14645	4855	10400
HGUS210-3	12	4⅜	9¼	4	8⅛	46-16d	16-16d	6840	14645	4855	10400
HGUS210-4	12	6⅜	9⅛	4	8⅛	46-16d	16-16d	6840	14645	4855	10400
HGUS212-4	12	6⅜	10⅝	4	10⅛	56-16d	20-16d	7640	14995	5425	10645
HGUS214-4	12	6⅜	12⅝	4	11⅛	66-16d	22-16d	10130	16400	7195	11645

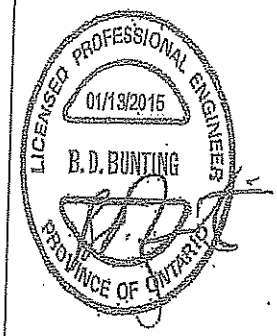
1. d_g is the distance from the seat of the hanger to the highest joist nail.


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580


Double Shear Nailing Side View. Do not bend tab back.

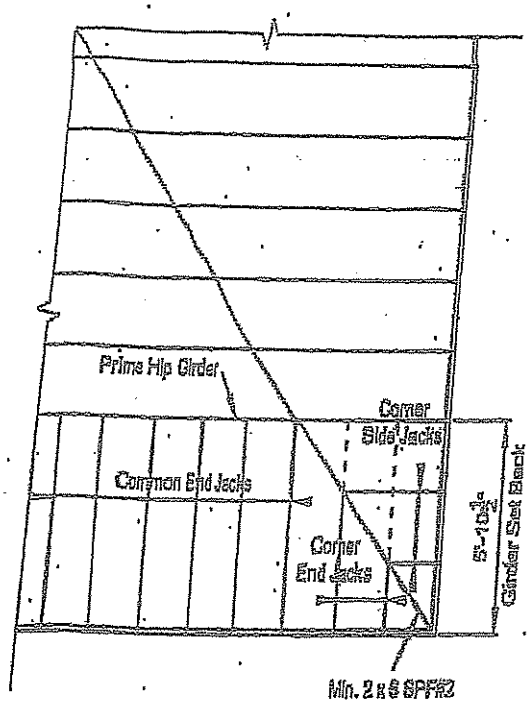

Double Shear Nailing Top View.



800.999.5099
www.strongtie.com

MICRO CITY

ENGINEERING SERVICES INC. TEL: (519) 287 - 2242



45° Hip End

LUMBER SPECIFICATION

TOP CHORD : 2x4 SPF#2

BOTTOM CHORD : 2x4 SPF#2

WEBS : 2x3 SPF#2

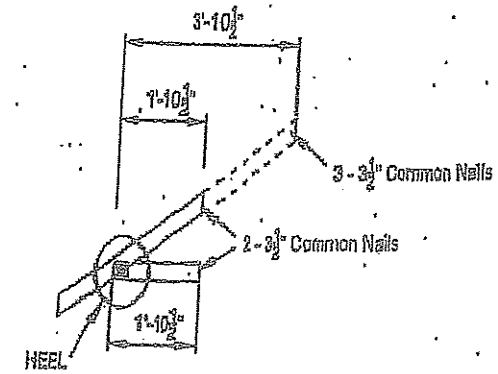
UNLESS OTHERWISE SHOWN

DESIGN LOAD:

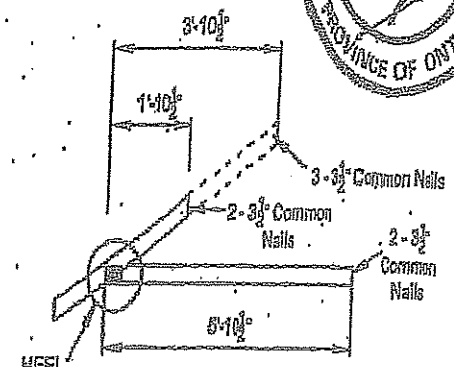
TOP CHORD LIVE LOAD	: 39.4 P.S.F.
TOP CHORD DEAD LOAD	: 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	: 10.5 P.S.F.
BOTTOM CHORD DEAD LOAD	: 7.0 P.S.F
TOTAL LOAD	= 59.9 P.S.F.

DWG NO TIM 2475.14
STRUCTURAL COMPONENT ONLY

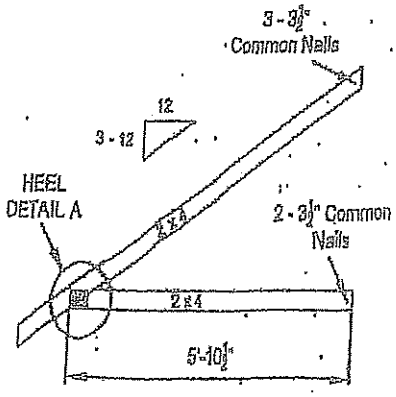
LICENSED PROFESSIONAL ENGINEER
1-20-14
S. KATSOULAKOS
PROVINCE OF ONTARIO



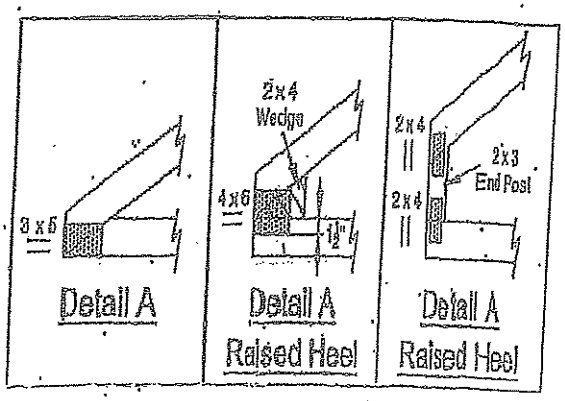
Corner Side Jacks



Corner End Jacks



Common End Jacks

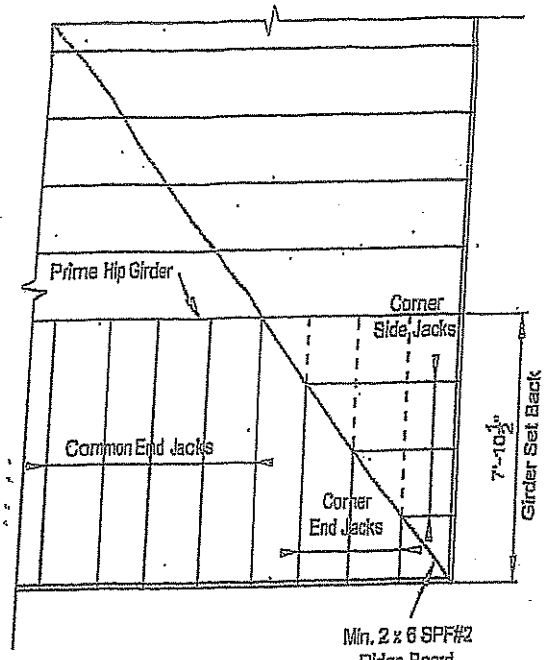


NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)

MICRO CITY
ENGINEERING SERVICES INC.
TEL: (519) 287.- 2242
R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

LUMBER SPECIFICATION

TOP CHORD : 2x4 SPF#2
BOTTOM CHORD : 2x4 SPF#2
WEBS : 2x3 SPF#2
UNLESS OTHERWISE SHOWN



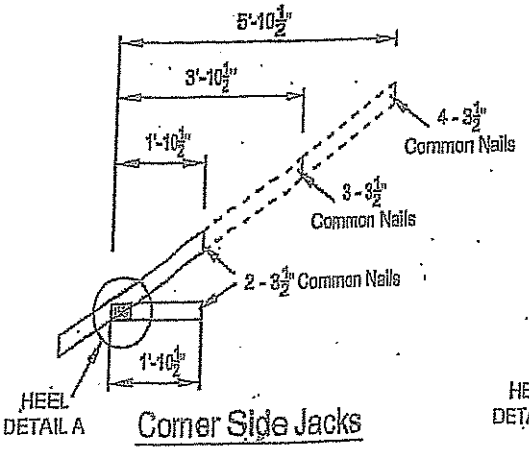
45° Hip End

DESIGN LOAD:

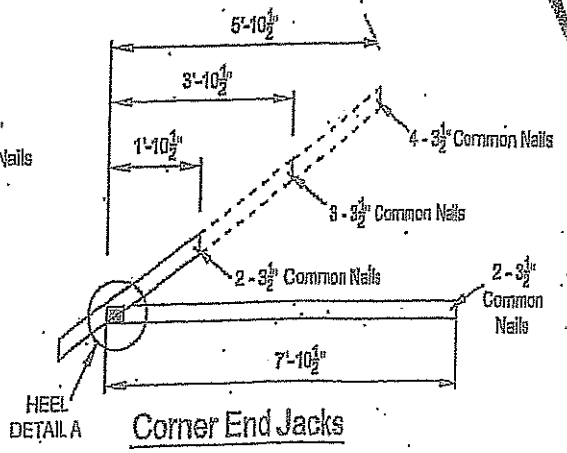
TOP CHORD LIVE LOAD	: 34.8 P.S.F.
TOP CHORD DEAD LOAD	: 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	: 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	: 7.0 P.S.F.
TOTAL LOAD	: 44.8 P.S.F.

DWG NO TRM 3503 14
STRUCTURAL COMPONENT ONLY

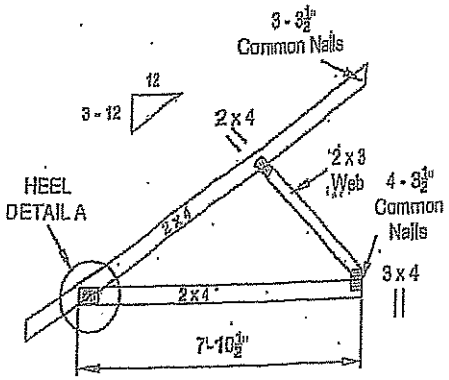
LICENSED PROFESSIONAL ENGINEER
1-2014
S. KATSOULAKOS
PROVINCE OF ONTARIO



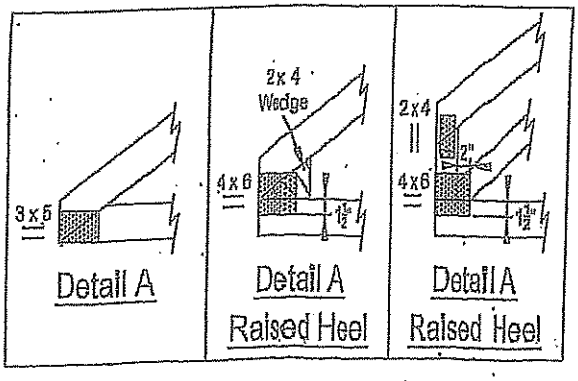
Corner Side Jacks



Corner End Jacks



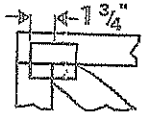
Common End Jacks



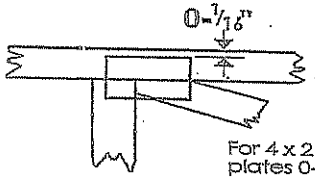
NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

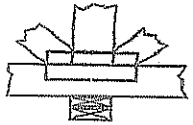
4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

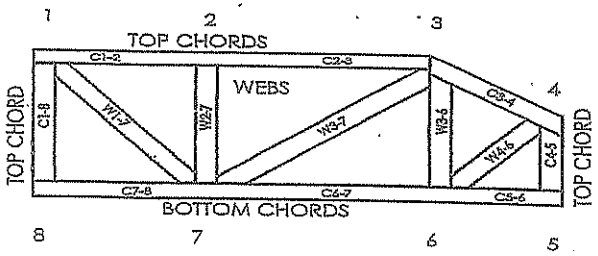
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses

DSB-89: *Design Standard for Bracing.*

BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

MiTek®

POWER TO PERFORM.™

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

Micro City Engineering Services Inc.
(BCIN: 26064; FIRM BCIN: 29991)
RR #1, Po Box 61
Glencoe, Ontario
N0L 1M0
(519) 287 - 2242; Fax: (519) 287 - 5750 (Call)

DWG NO T&M 24744
STRUCTURAL
COMPONENT ONLY

Responsibilities:

Micro City Engineering Services is responsible for the design of trusses as individual components.

It is the responsibilities of others to ascertain that the design loads utilized on this (these) drawing(s) meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdiction over such decisions.

All dimensions are to be verified by the owner, contractor, architect, or other authority having input over such decisions prior to truss component manufacture. At no time shall Micro City Engineering Services Inc. or its employees be responsible for dimension errors.

Micro City Engineering Services Inc. bears no responsibility for the erection of any truss components. Persons erecting truss components are cautioned to seek professional advice regarding temporary and permanent bracing systems and to be totally familiar with all aspects of truss erection prior to proceeding on any truss component erection job. Any bracing shown on Micro City Engineering Services Inc. or Tamarack Roof Trusses Inc. sealed or unsealed truss component drawings is specified for the single truss component in question and is identified as an integral part of the design for that particular truss component but is not meant to represent the only required bracing for that particular truss component when installed as a component in a series of truss components in a roof truss system.

~~It is the truss manufacturer's responsibility to ensure that trusses are manufactured in accordance with Micro City Engineering~~ Services Inc. specifications outlined below:

SPECIFICATIONS:

Truss components sealed by Micro City Engineering Services Inc. must conform to the relevant sections of the current Building Code of Ontario and Canada (Part 4 or Part 9) or the current Farm Building Code of Canada in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the Truss Plate Institute of Canada (TPIC). All unit lumber and nailing stresses identified on truss component design drawings and/or used in the design of individual truss components shall conform to the current CSA Wood Design standard identified in the current Building Code and TPIC Design Standards.

The lumber used to manufacture any truss component is to conform to the specified size and grade identified on the truss drawing.

The lumber used in the manufacture of any truss component is not to exceed 19% during its service use unless specifically noted on the truss drawing.

The lumber used in the manufacture of any truss component is not to be treated with any chemicals during its service life unless specifically noted on the truss drawing.

Connector plates shall be applied to both faces of the truss component at each joint and shall be positioned exactly as specified.

The top chord of any truss component is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the sealed truss component drawing but not exceeding 24" o/c (Part 9 design) and not exceeding 48" o/c (Part 4 or Agricultural design).

When a truss component is to be installed with no rigid ceiling attached directly to the bottom chord, then the bottom chord is to be laterally braced at intervals not exceeding 3m (or 10'-0").

All sealed or unsealed truss component drawings provided by Micro City Engineering Services Inc. Or Tamarack Roof Trusses Inc. should be read in conjunction with the following:

Warning-Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473C rev 10-'08 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer - not the truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component Safety Information available from the Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA, 22314.