

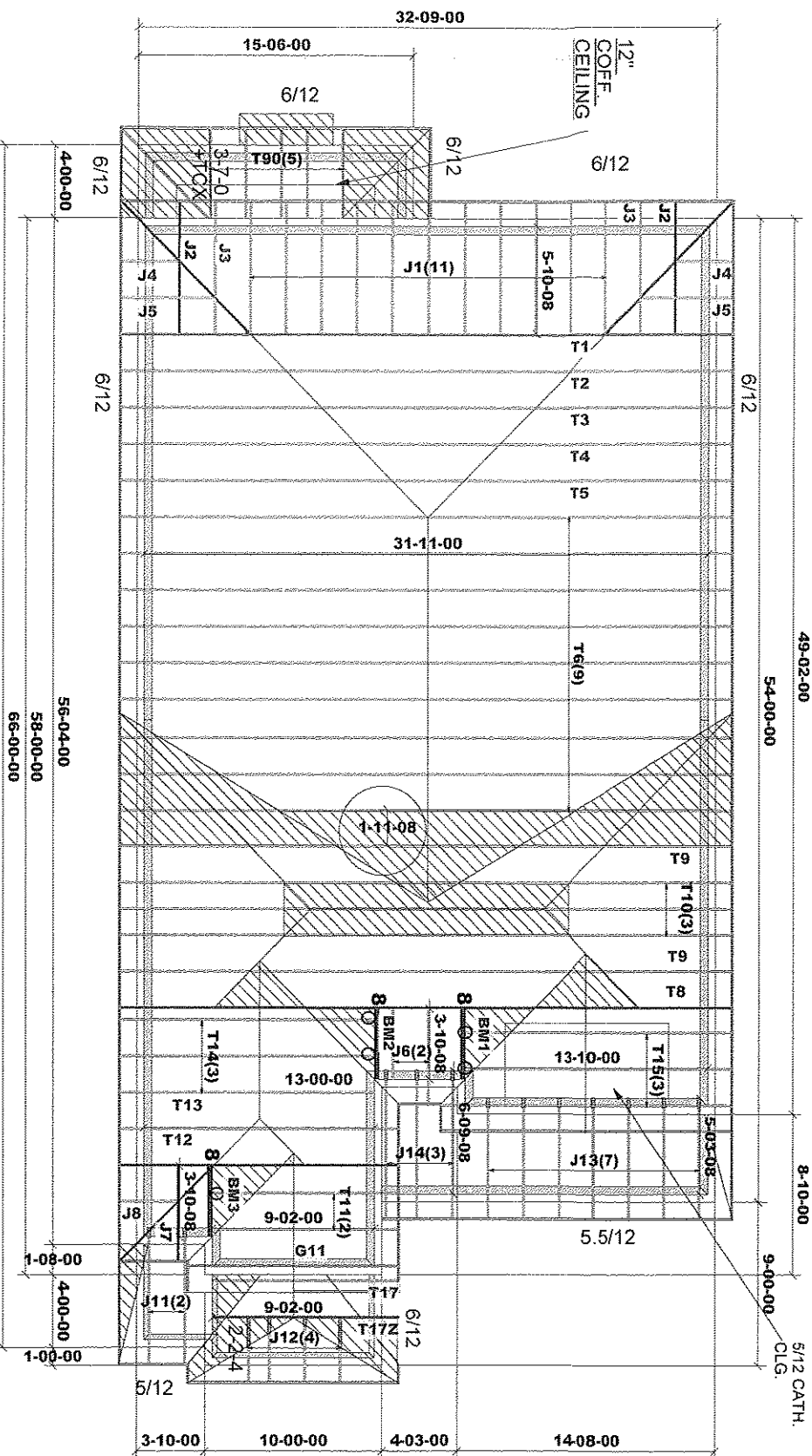
M10,962



Job Track: **43954**
Layout ID: **282110**
Plan Log: **92173**

Builder / Location: **GREENPARK HOMES / WATERDOWN**
Project: **RUSSEL GARDENS PHASE 2**
Date: **8/29/2017** Designer: **JG/S V E**

Model / Elevation: **ROSEWOOD 7 / 1**
THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.



12" FINISH O.H.
R.T.M.C.
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART 9 OF THE OBC, LATEST EDITION. ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"x4" SPF @ 24" o.c. WITH A 2"x4" SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT. POSTS LONGER THAN 6' TO BE Laterally BRACED SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN CONFORMS WITH THE RELEVANT SECTION OF THE LATEST EDITION OF O.B.C. PART 9

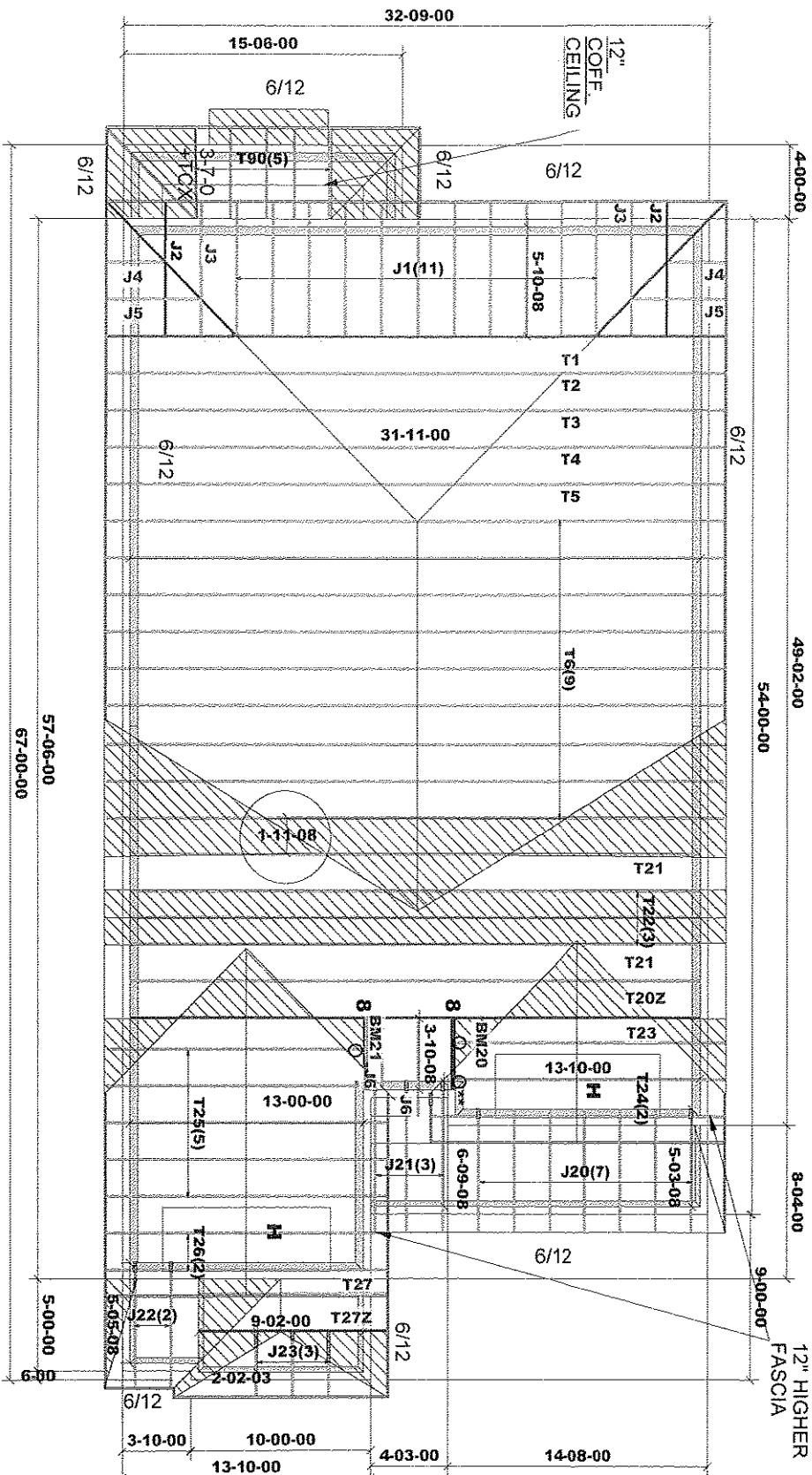
DESIGN LOADS:
GROUND SNOW LOAD
S_s = 1.5 kPa
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

DENOTES CONVENTIONAL FRAMING

HARDWARE
LUS24(0)
LUS24-2(8)

10/12 ROOF PITCH
BEAMS/BM: 2-2X8
UNLESS NOTED

T-170329



12" FINISH O.H.
RT/MC
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART 9 OF THE OBC LATEST EDITION ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"x4"SPF @24"OC. WITH A 2"x4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT. POSTS LONGER THAN 6' TO BE Laterally BRACED SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN CONFORMS WITH THE RELEVANT SECTION OF THE LATEST EDITION OF O.B.C. PART 9

DESIGN LOADS:

GROUND SNOW LOAD
S_s = 1.5 kPa
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

DEMOTES
CONVENTIONAL
FRAMING

HARDWARE
LUS24(0)
LUS24-2(9)

H-12" HIGH COFF.

10/12 ROOF PITCH
BEAMS(BM): 2-2X8
UNLESS NOTED

T-170329

M10.982



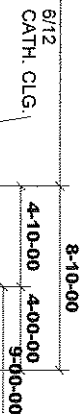
Job Track: **43954**
Layout ID: **282111**
Plan Log: **92173**

Builder / Location: **GREENPARK HOMES / WATERDOWN**
Project: **RUSSEL GARDENS PHASE 2**
Date: **8/29/2017** Designer: **JG/S V/E**

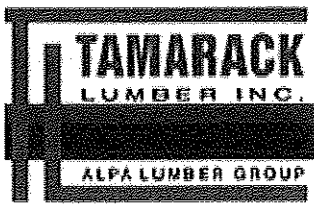
Model / Elevation: **ROSEWOOD 7/2**

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

Milk, ver 7.5.0



T-170329



Delivery Shiplist

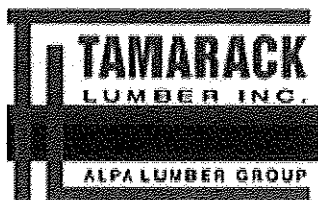
DATE	08/29/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 282110 LOCATION: WATERDOWN
 BUILDER: GREENPARK - RUSSELL GARDENS PH 2 SUB-BUILDER:
 MODEL: ROSEWOOD 7 ELEVATION: 1

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY PLY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
						TOP	BOT					
	1	T1 HIP GIRDER	6.00 0.00	31-11-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-02-00 01-02-00	170.72 104.34		
	1	T2 HIP	6.00 0.00	31-11-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	127.46 79.83		
	1	T3 HIP	6.00 0.00	31-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	130.07 81.34		
	1	T4 HIP	6.00 0.00	31-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	131.31 81.33		
	1	T5 HIP	6.00 0.00	31-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	132.23 83.17		
	9	T6 COMMON	6.00 0.00	31-11-00	09-01-12	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	1171.80 724.50		
	1	T8 HIP GIRDER	10.00 0.00	31-11-00	04-10-07	2 X 6	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	191.62 113.34		
	2	T9 HIP	10.00 0.00	31-11-00	06-06-07	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	271.96 168.34		
	3	T10 HIP	10.00 0.00	31-11-00	08-02-07	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	459.33 291.00		
	1	T12 HIP GIRDER	10.00 0.00	13-00-00	04-10-07	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	60.63 38.67		
	1	T13 HIP	10.00 0.00	13-00-00	06-06-07	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	64.75 41.50		
	3	T14 COMMON	10.00 0.00	13-00-00	07-00-11	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	186.33 118.50		
	3	T15 ROOF	10.00 5.00	13-10-00	07-04-14	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	192.57 128.49		
	2	T11 COMMON	10.00 0.00	09-02-00	05-05-08	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	84.44 55.00		
	1	G11 GABLE	10.00 0.00	09-02-00	05-05-08	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	42.85 28.50		
	1	T17 COMMON	6.00 0.00	09-02-00	02-08-15	2 X 4	2 X 4	01-03-08 01-03-08	00-05-07 00-05-07	27.50 17.50		
	1	T17Z COMMON	6.00 0.00	09-02-00	02-08-15	2 X 4	2 X 4	01-03-08 01-03-08	00-05-07 00-05-07	27.50 17.50		
	5	T90 MONOPITCH	6.00 0.00	03-07-00	03-04-02	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 02-04-02	106.60 76.65		



Delivery Shiplist

DATE	08/29/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 282110 LOCATION: WATERDOWN
 BUILDER: GREENPARK - RUSSELL GARDENS PH 2 SUB-BUILDER:
 MODEL: ROSEWOOD 7 ELEVATION: 1

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	11	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	184.69 117.37		
	2	J2 JACK-OPEN	6.00 0.00	05-10-08	02-00-12	2 X 4	2 X 4	01-03-08 -04-01-01	01-02-00 00-03-08	32.32 20.66		
	2	J3 JACK-OPEN	6.00 0.00	05-10-08	03-00-12	2 X 4	2 X 4	01-03-08 -02-01-01	01-02-00 00-03-08	37.42 23.34		
	2	J4 JACK-OPEN	6.00 0.00	01-10-08	02-00-12	2 X 4	2 X 4	01-03-08 -00-01-01	01-02-00 00-03-08	16.86 11.34		
	2	J5 JACK-OPEN	6.00 0.00	01-10-08	03-00-12	2 X 4	2 X 4	01-03-08 01-10-15	01-02-00 00-04-11	21.96 14.00		
	2	J6 JACK-OPEN	10.00 0.00	03-10-08	04-10-07	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 04-10-07	27.50 17.34		
	1	J7 JACK-OPEN	10.00 0.00	03-10-08	03-01-09	2 X 4	2 X 4	01-03-08 -02-01-01	01-07-11 00-03-08	13.73 9.33		
	1	J8 JACK-OPEN	10.00 0.00	01-10-08	03-01-09	2 X 4	2 X 4	01-03-08 -00-01-01	01-07-11 00-03-08	9.99 7.00		
	2	J11 JACK-OPEN	5.00 0.00	06-01-08	02-10-11	2 X 4	2 X 4	01-03-08 00-00-00	00-04-01 02-10-11	32.56 21.34		
	4	J12 JACK-OPEN	10.00 0.00	02-02-04	02-09-00	2 X 4	2 X 4	01-03-08 00-00-00	00-11-02 02-09-00	34.00 24.00		
	7	J13 JACK-OPEN	5.50 0.00	05-03-08	02-09-03	2 X 4	2 X 4	01-03-08 00-00-00	00-04-02 02-09-03	101.08 65.31		
	3	J14 JACK-OPEN	5.50 0.00	06-09-08	03-05-07	2 X 4	2 X 4	01-03-08 00-00-00	00-04-02 03-05-07	60.63 38.49		

TOTAL # TRUSS= 77.00

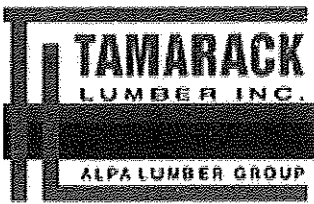
TOTAL BFT OF ALL TRUSSES=

2619.02 BFT. TOTAL WEIGHT OF ALL TRUSSES= 4152.41 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	LUS24-2	
5	Hangers	LUS24	

TOTAL # ITEMS= 8.00



Delivery Shiplist

DATE	08/29/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 282111 LOCATION: WATERDOWN
 BUILDER: GREENPARK - RUSSELL GARDENS PH 2 SUB-BUILDER:
 MODEL: ROSEWOOD 7 ELEVATION: 2

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	T1 HIP GIRDER	6.00 0.00	31-11-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-02-00 01-02-00	170.72 104.34		
	1	T2 HIP	6.00 0.00	31-11-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	127.46 79.83		
	1	T3 HIP	6.00 0.00	31-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	130.07 81.34		
	1	T4 HIP	6.00 0.00	31-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	131.31 81.33		
	1	T5 HIP	6.00 0.00	31-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	132.23 83.17		
	9	T6 COMMON	6.00 0.00	31-11-00	09-01-12	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	1171.80 724.50		
	1	T20Z FLAT GIRDER	0.00 0.00	31-11-00	04-10-07	2 X 6	2 X 6	00-00-00 00-00-00	04-10-07 04-10-07	180.87 106.00		
	2	T21 FLAT	0.00 0.00	31-11-00	06-06-07	2 X 4	2 X 4	00-00-00 00-00-00	06-06-07 06-06-07	280.96 171.66		
	3	T22 FLAT	0.00 0.00	31-11-00	08-02-07	2 X 4	2 X 4	00-00-00 00-00-00	08-02-07 08-02-07	464.22 285.00		
	1	T23 COMMON	10.00 0.00	13-10-00	08-04-14	2 X 4	2 X 4	01-03-08 01-03-08	02-07-11 02-07-11	71.17 44.00		
	2	T24 ROOF	10.00 0.00	13-10-00	08-04-14	2 X 4	2 X 4	01-03-08 01-03-08	02-07-11 02-07-11	153.08 99.66		
	5	T25 COMMON	10.00 0.00	13-00-00	08-00-11	2 X 4	2 X 4	01-03-08 01-03-08	02-07-11 02-07-11	339.30 216.65		
	2	T26 ROOF	10.00 0.00	13-00-00	08-00-11	2 X 4	2 X 4	01-03-08 01-03-08	02-07-11 02-07-11	143.40 95.00		
	1	T27 COMMON	6.00 0.00	09-02-00	02-07-11	2 X 4	2 X 4	01-03-08 01-03-08	00-04-03 00-04-03	27.41 17.50		
	1	T27Z COMMON	6.00 0.00	09-02-00	02-07-11	2 X 4	2 X 4	01-03-08 01-03-08	00-04-03 00-04-03	27.41 17.50		
	5	T90 MONOPITCH	6.00 0.00	03-07-00	03-04-02	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 02-04-02	106.60 76.65		
	11	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	184.69 117.37		
	2	J2 JACK-OPEN	6.00 0.00	05-10-08	02-00-12	2 X 4	2 X 4	01-03-08 -04-01-01	01-02-00 00-03-08	32.32 20.66		



Delivery Shiplist

DATE	08/29/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 282111 LOCATION: WATERDOWN
 BUILDER: GREENPARK - RUSSELL GARDENS PH 2 SUB-BUILDER:
 MODEL: ROSEWOOD 7 ELEVATION: 2

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	2	J3 JACK-OPEN	6.00 0.00	05-10-08	03-00-12	2 X 4	2 X 4	01-03-08 -02-01-01	01-02-00 00-03-08	37.42 23.34		
	2	J4 JACK-OPEN	6.00 0.00	01-10-08	02-00-12	2 X 4	2 X 4	01-03-08 -00-01-01	01-02-00 00-03-08	16.86 11.34		
	2	J5 JACK-OPEN	6.00 0.00	01-10-08	03-00-12	2 X 4	2 X 4	01-03-08 01-10-15	01-02-00 00-04-11	21.96 14.00		
	2	J6 JACK-OPEN	10.00 0.00	03-10-08	04-10-07	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 00-05-13	27.50 17.34		
	7	J20 JACK-OPEN	6.00 0.00	05-03-08	02-11-15	2 X 4	2 X 4	01-03-08 00-00-00	00-04-03 02-11-15	102.13 65.31		
	3	J21 JACK-OPEN	6.00 0.00	06-09-08	03-08-15	2 X 4	2 X 4	01-03-08 00-00-00	00-04-03 03-08-15	61.26 38.49		
	2	J22 JACK-OPEN	6.00 0.00	05-05-08	03-00-15	2 X 4	2 X 4	01-03-08 00-00-00	00-04-03 03-00-15	30.00 18.66		
	3	J23 JACK-OPEN	10.00 0.00	02-02-04	02-07-11	2 X 4	2 X 4	01-03-08 00-00-00	00-09-13 02-07-11	25.14 18.00		

TOTAL # TRUSS= 73.00

TOTAL BFT OF ALL TRUSSES=

2628.64 BFT. TOTAL WEIGHT OF ALL TRUSSES= 4197.29 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	LUS24	
2	Hangers	LUS24-2	

TOTAL # ITEMS= 5.00



Delivery Shiplist

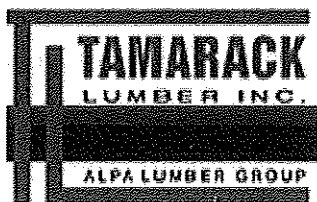
DATE	08/29/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 282113 LOCATION: WATERDOWN
 BUILDER: GREENPARK - RUSSELL GARDENS PH 2 SUB-BUILDER:
 MODEL: ROSEWOOD 7 ELEVATION: 3

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY PLY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
						TOP	BOT					
	1	T1 HIP GIRDER	6.00 0.00	31-11-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-02-00 01-02-00	170.72 104.34		
	1	T2 HIP	6.00 0.00	31-11-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	127.46 79.83		
	1	T3 HIP	6.00 0.00	31-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	130.07 81.34		
	1	T4 HIP	6.00 0.00	31-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	131.31 81.33		
	1	T5 HIP	6.00 0.00	31-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	132.23 83.17		
	9	T6 COMMON	6.00 0.00	31-11-00	09-01-12	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	1171.80 724.50		
	1	T8Z HIP GIRDER	10.00 0.00	31-11-00	04-10-07	2 X 6	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	191.62 113.34		
	2	T9 HIP	10.00 0.00	31-11-00	06-06-07	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	271.96 168.34		
	3	T10 HIP	10.00 0.00	31-11-00	08-02-07	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	459.33 291.00		
	2	T28 ROOF	10.00 6.00	13-10-00	07-04-14	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	128.36 85.66		
	1	T29 COMMON	10.00 0.00	13-10-00	07-04-14	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	65.43 42.17		
	1	T12Z HIP GIRDER	10.00 0.00	13-00-00	04-10-07	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	60.63 38.67		
	1	T13 HIP	10.00 0.00	13-00-00	06-06-07	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	64.75 41.50		
	3	T14 COMMON	10.00 0.00	13-00-00	07-00-11	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	186.33 118.50		
	1	T11 COMMON	10.00 0.00	09-02-00	05-05-08	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	42.22 27.50		
	2	T30 SCISSORS	10.00 6.00	09-02-00	05-05-08	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	87.30 55.66		
	5	T90 MONOPITCH	6.00 0.00	03-07-00	03-04-02	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 02-04-02	106.60 76.65		
	11	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	184.69 117.37		



Delivery Shiplist

DATE	08/29/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 282113 LOCATION: WATERDOWN
 BUILDER: GREENPARK - RUSSELL GARDENS PH² SUB-BUILDER:
 MODEL: ROSEWOOD 7 ELEVATION: 3

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	2	J2 JACK-OPEN	6.00 0.00	05-10-08	02-00-12	2 X 4	2 X 4	01-03-08 -04-01-01	01-02-00 00-03-08	32.32 20.66		
	2	J3 JACK-OPEN	6.00 0.00	05-10-08	03-00-12	2 X 4	2 X 4	01-03-08 -02-01-01	01-02-00 00-03-08	37.42 23.34		
	2	J4 JACK-OPEN	6.00 0.00	01-10-08	02-00-12	2 X 4	2 X 4	01-03-08 -00-01-01	01-02-00 00-03-08	16.86 11.34		
	2	J5 JACK-OPEN	6.00 0.00	01-10-08	03-00-12	2 X 4	2 X 4	01-03-08 01-10-15	01-02-00 00-04-11	21.96 14.00		
	2	J6 JACK-OPEN	10.00 0.00	03-10-08	04-10-07	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 04-10-07	27.50 17.34		
	1	J7 JACK-OPEN	10.00 0.00	03-10-08	03-01-09	2 X 4	2 X 4	01-03-08 -02-01-01	01-07-11 00-03-08	13.73 9.33		
	1	J8 JACK-OPEN	10.00 0.00	01-10-08	03-01-09	2 X 4	2 X 4	01-03-08 -00-01-01	01-07-11 00-03-08	9.99 7.00		
	7	J20 JACK-OPEN	6.00 0.00	05-03-08	02-11-15	2 X 4	2 X 4	01-03-08 00-00-00	00-04-03 02-11-15	102.13 65.31		
	3	J21 JACK-OPEN	6.00 0.00	06-09-08	03-08-15	2 X 4	2 X 4	01-03-08 00-00-00	00-04-03 03-08-15	61.26 38.49		

TOTAL # TRUSS= 69.00

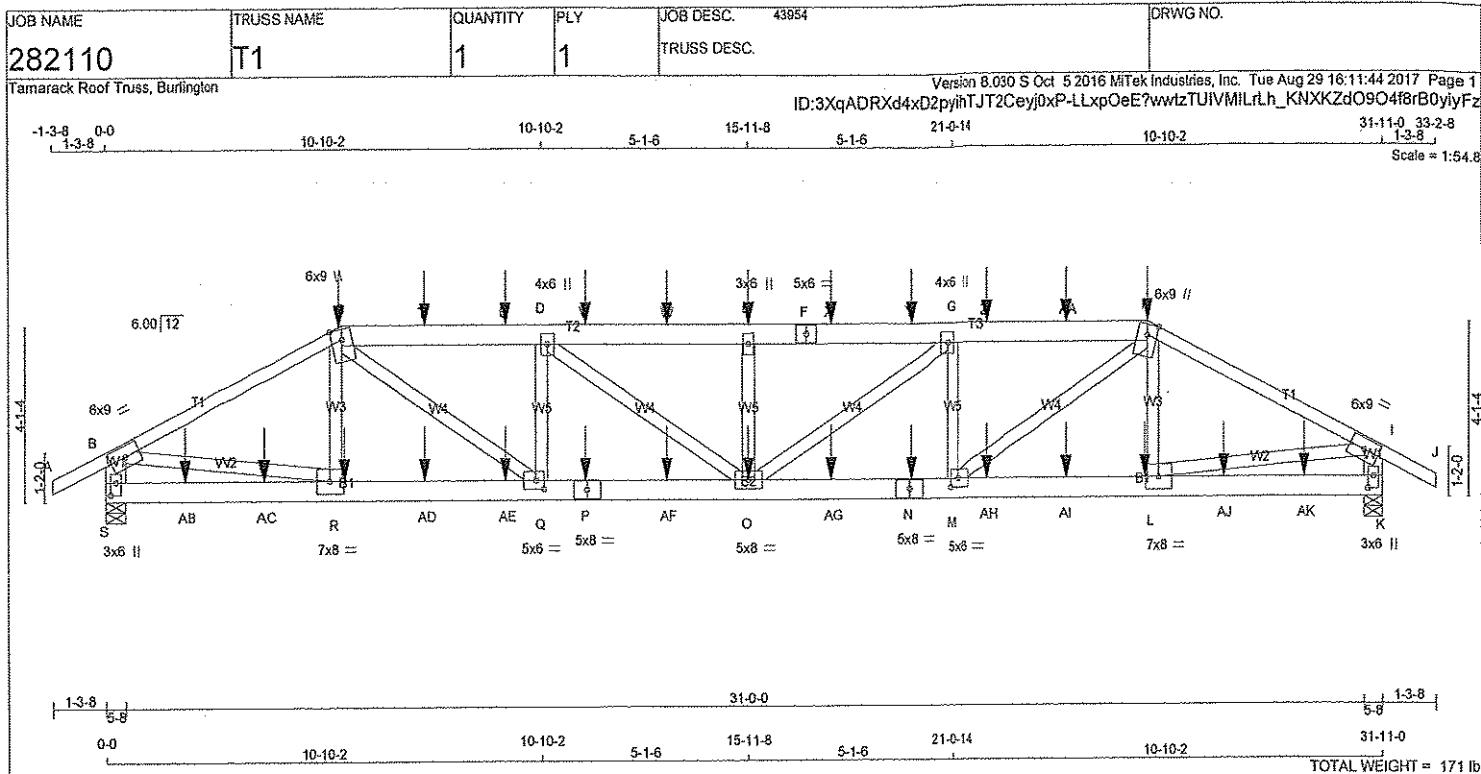
TOTAL BFT OF ALL TRUSSES=

2537.68 BFT. TOTAL WEIGHT OF ALL TRUSSES= 4035.98 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	LUS24-2	
5	Hangers	LUS24	

TOTAL # ITEMS= 8.00



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	1650F 1.5E
C - F	2x6	DRY	No.2
F - H	2x6	DRY	No.2
H - J	2x4	DRY	1650F 1.5E
S - B	2x6	DRY	No.2
K - I	2x6	DRY	No.2
S - P	2x6	DRY	No.2
P - N	2x6	DRY	No.2
N - K	2x6	DRY	No.2
ALL WEBS EXCEPT	2x4	DRY	No.2

ORY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-1	MT20	6.0	9.0		
C	TTWW+m	MT20	6.0	9.0	Edge	
D	TMVW+1	MT20	4.0	6.0		
E	TMVW+w	MT20	3.0	6.0		
F	TS-1	MT20	5.0	6.0		
G	TMVW+1	MT20	4.0	6.0		
H	TTWW+m	MT20	6.0	9.0	Edge	
I	TMVW-1	MT20	6.0	9.0		
K	BMV1+p	MT20	3.0	6.0	3.50	1.50
L	BMVW-1	MT20	7.0	8.0		
M	BMVW-1	MT20	5.0	6.0	2.50	2.50
N	BS-1	MT20	5.0	8.0		
O	BMVW-1	MT20	5.0	8.0		
P	BS-1	MT20	5.0	8.0		
Q	BMVW-1	MT20	5.0	6.0	2.50	2.50
R	BMVW-1	MT20	7.0	8.0		
S	BMV1+p	MT20	3.0	6.0	3.50	1.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	BRG
S	3151	0	3151	0	0	5-8	5-8	5-8	
K	3151	0	3151	0	0	5-8	5-8	5-8	

UNFACTORED REACTIONS

1ST CASE		MAX / MIN COMPONENT REACTIONS		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	COMBINED	SNOW
S	2574	1448 / 0	572 / 0	0 / 0	0 / 0	554 / 0	0 / 0	2574	1448 / 0
K	2574	1448 / 0	572 / 0	0 / 0	0 / 0	554 / 0	0 / 0	2574	1448 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.95 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WIND		DEAD		SOIL	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED (LC1)	MAX. FACTORED (LC2)	MAX. FACTORED (LC3)	MAX. FACTORED (LC4)	MAX. FACTORED (LC5)	MAX. FACTORED (LC6)	MAX. FACTORED (LC7)
FR-TO									
A-B	0 / 26	-84.3	-84.3	0.10 (1)	10.00	R-C	-285 / 246	0.05 (1)	
B-C	-4736 / 0	-84.3	-84.3	0.88 (1)	3.04	L-H	-285 / 246	0.05 (1)	
C-T	-6244 / 0	-84.3	-84.3	0.50 (1)	3.11	B-R	0 / 4284	0.78 (1)	
T-U	-6244 / 0	-84.3	-84.3	0.50 (1)	3.11	L-I	0 / 4284	0.78 (1)	
U-D	-6244 / 0	-84.3	-84.3	0.50 (1)	3.11	M-H	0 / 2472	0.44 (1)	
D-V	-6873 / 0	-84.3	-84.3	0.52 (1)	2.95	C-Q	0 / 2472	0.44 (1)	
V-W	-6873 / 0	-84.3	-84.3	0.52 (1)	2.95	M-G	-1232 / 0	0.21 (1)	
W-E	-6873 / 0	-84.3	-84.3	0.52 (1)	2.95	Q-D	-1232 / 0	0.21 (1)	
E-F	-6873 / 0	-84.3	-84.3	0.52 (1)	2.95	O-G	0 / 1773	0.14 (1)	
F-X	-6873 / 0	-84.3	-84.3	0.52 (1)	2.95	D-O	0 / 1773	0.14 (1)	
X-Y	-6873 / 0	-84.3	-84.3	0.52 (1)	2.95	O-E	-559 / 0	0.11 (1)	
Y-G	-6873 / 0	-84.3	-84.3	0.52 (1)	2.95				
G-Z	-6244 / 0	-84.3	-84.3	0.50 (1)	3.11				
Z-AA	-6244 / 0	-84.3	-84.3	0.50 (1)	3.11				
AA-H	-6244 / 0	-84.3	-84.3	0.50 (1)	3.11				
H-I	-4736 / 0	-84.3	-84.3	0.88 (1)	3.04				
I-J	0 / 26	-84.3	-84.3	0.10 (1)	10.00				
S-B	-3034 / 0	0.0	0.0	0.21 (1)	6.00				
K-I	-3034 / 0	0.0	0.0	0.21 (1)	6.00				
S-AB	0 / 0	-28.0	-28.0	0.20 (2)	10.00				
AB-AC	0 / 0	-28.0	-28.0	0.20 (2)	10.00				
AC-R	0 / 0	-28.0	-28.0	0.20 (2)	10.00				
R-AD	0 / 4257	-28.0	-28.0	0.63 (1)	10.00				
AD-AE	0 / 4257	-28.0	-28.0	0.63 (1)	10.00				
AE-Q	0 / 4257	-28.0	-28.0	0.63 (1)	10.00				
Q-P	0 / 6244	-28.0	-28.0	0.92 (1)	10.00				
P-AF	0 / 6244	-28.0	-28.0	0.92 (1)	10.00				
AF-O	0 / 6244	-28.0	-28.0	0.92 (1)	10.00				
O-AG	0 / 6244	-28.0	-28.0	0.92 (1)	10.00				
AG-N	0 / 6244	-28.0	-28.0	0.92 (1)	10.00				
N-M	0 / 6244	-28.0	-28.0	0.92 (1)	10.00				
M-AH	0 / 4257	-28.0	-28.0	0.63 (1)	10.00				
AH-AI	0 / 4257	-28.0	-28.0	0.63 (1)	10.00				
AI-L	0 / 4257	-28.0	-28.0	0.63 (1)	10.00				
L-AJ	0 / 0	-28.0	-28.0	0.20 (2)	10.00				
AJ-AK	0 / 0	-28.0	-28.0	0.20 (2)	10.00				
AK-K	0 / 0	-28.0	-28.0	0.20 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (1.06")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.34")
ALLOWABLE DEFL.(TL) = $L/360$ (1.06")
CALCULATED VERT. DEFL.(TL) = $L/709$ (0.54")

CSI: TC=0.88 (B-C:1), BC=0.92 (O-Q:1), WB=0.76 (B-R:1), SSI=0.26 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

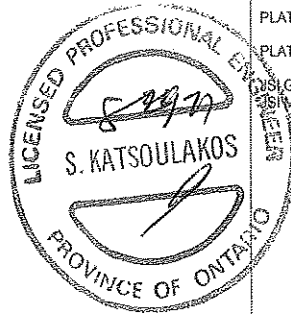
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	618	354	1687
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

GRIP=0.88 (I) (INPUT = 0.80)
METAL=1.00 (N) (INPUT = 1.00)



DRWG NO. TAM 43518-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
282110	T1	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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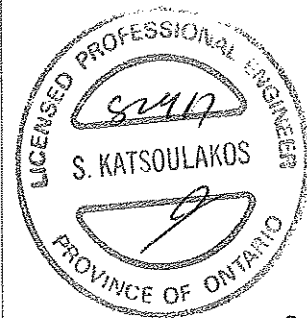
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HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 472.3 lbs FACTORED DOWN AT 5-10-8, 101.4 lbs FACTORED DOWN AT 7-11-4, 101.4 lbs FACTORED DOWN AT 9-11-4, 101.4 lbs FACTORED DOWN AT 11-11-4, 101.4 lbs FACTORED DOWN AT 13-11-4, 101.4 lbs FACTORED DOWN AT 15-11-8, 101.4 lbs FACTORED DOWN AT 17-11-12, 101.4 lbs FACTORED DOWN AT 19-11-12, 101.4 lbs FACTORED DOWN AT 21-11-12, AND 101.4 lbs FACTORED DOWN AT 23-11-12, AND 472.3 lbs FACTORED DOWN AT 26-0-8 ON TOP CHORD, AND 84.6 lbs FACTORED DOWN AT 1-11-4, 84.6 lbs FACTORED DOWN AT 3-11-4, 69.9 lbs FACTORED DOWN AT 5-11-4, 69.9 lbs FACTORED DOWN AT 7-11-4, 69.9 lbs FACTORED DOWN AT 9-11-4, 69.9 lbs FACTORED DOWN AT 11-11-4, 69.9 lbs FACTORED DOWN AT 13-11-4, 69.9 lbs FACTORED DOWN AT 15-11-8, 69.9 lbs FACTORED DOWN AT 17-11-12, 69.9 lbs FACTORED DOWN AT 19-11-12, 69.9 lbs FACTORED DOWN AT 21-11-12, 69.9 lbs FACTORED DOWN AT 23-11-12, 69.9 lbs FACTORED DOWN AT 25-11-12, AND 84.6 lbs FACTORED DOWN AT 27-11-12, AND 84.6 lbs FACTORED DOWN AT 29-11-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

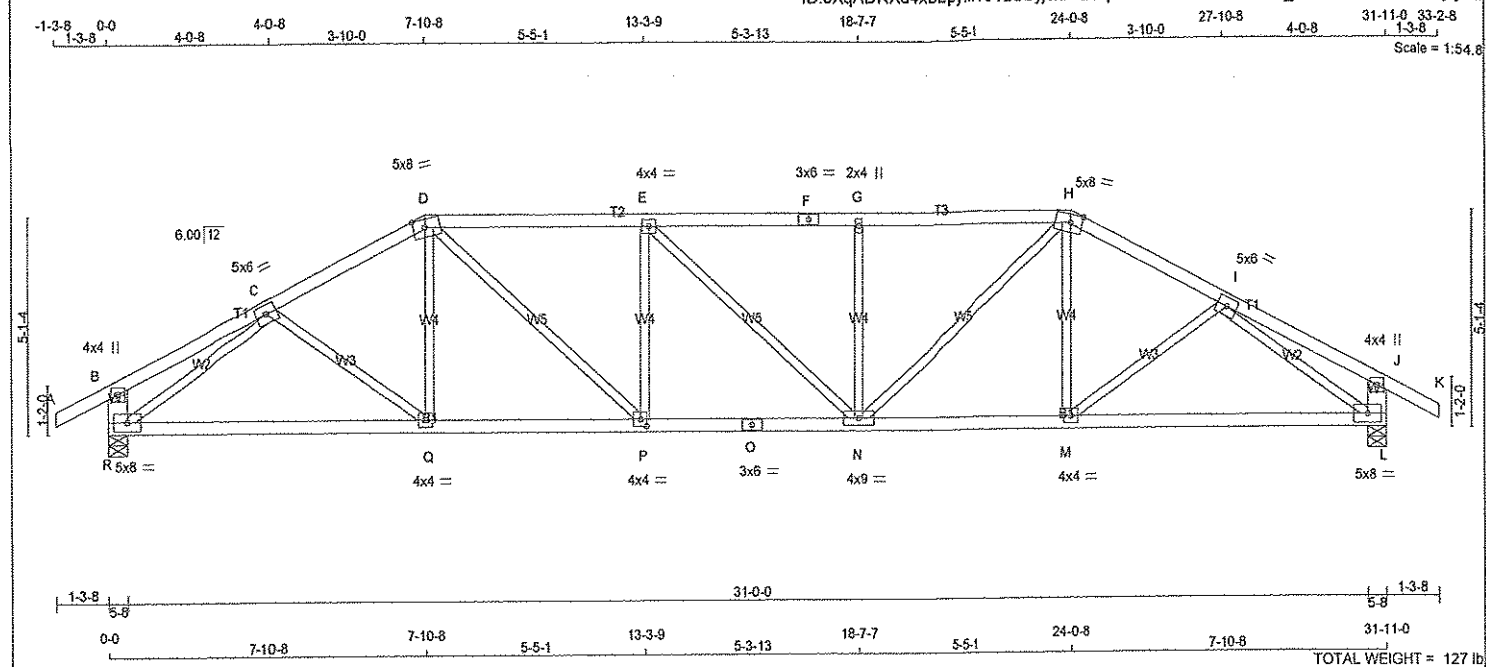
FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	5-10-8	-472	-472	---	FRONT	VERT	TOTAL
E	15-11-8	-101	-101	---	FRONT	VERT	TOTAL
H	26-0-8	-472	-472	---	FRONT	VERT	TOTAL
L	25-11-12	-40	-70	---	FRONT	VERT	TOTAL
N	19-11-12	-40	-70	---	FRONT	VERT	TOTAL
O	15-11-8	-40	-70	---	FRONT	VERT	TOTAL
P	11-11-4	-40	-70	---	FRONT	VERT	TOTAL
R	5-11-4	-40	-70	---	FRONT	VERT	TOTAL
T	7-11-4	-101	-101	---	FRONT	VERT	TOTAL
U	9-11-4	-101	-101	---	FRONT	VERT	TOTAL
V	11-11-4	-101	-101	---	FRONT	VERT	TOTAL
W	13-11-4	-101	-101	---	FRONT	VERT	TOTAL
X	17-11-12	-101	-101	---	FRONT	VERT	TOTAL
Y	19-11-12	-101	-101	---	FRONT	VERT	TOTAL
Z	21-11-12	-101	-101	---	FRONT	VERT	TOTAL
AA	23-11-12	-101	-101	---	FRONT	VERT	TOTAL
AB	1-11-4	-48	-85	---	FRONT	VERT	TOTAL
AC	3-11-4	-48	-85	---	FRONT	VERT	TOTAL
AD	7-11-4	-40	-70	---	FRONT	VERT	TOTAL
AE	9-11-4	-40	-70	---	FRONT	VERT	TOTAL
AF	13-11-4	-40	-70	---	FRONT	VERT	TOTAL
AG	17-11-12	-40	-70	---	FRONT	VERT	TOTAL
AH	21-11-12	-40	-70	---	FRONT	VERT	TOTAL
AI	23-11-12	-40	-70	---	FRONT	VERT	TOTAL
AJ	27-11-12	-48	-85	---	FRONT	VERT	TOTAL
AK	29-11-12	-48	-85	---	FRONT	VERT	TOTAL



P22

SWG NO. TAM 43518-17
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
D - F	2x4	DRY No.2	SPF
F - H	2x4	DRY No.2	SPF
H - K	2x4	DRY No.2	SPF
R - B	2x6	DRY No.2	SPF
L - J	2x6	DRY No.2	SPF
R - O	2x4	DRY No.2	SPF
O - L	2x4	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMVW-t	MT20	5.0	6.0		
D	TTWW-m	MT20	5.0	8.0	2.25	3.50
E	TMVW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMVW-w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	8.0	2.25	3.50
I	TMVW-t	MT20	5.0	6.0		
J	TMV+p	MT20	4.0	4.0		
L	BMVW-t	MT20	5.0	8.0		
M	BMVW-t	MT20	4.0	4.0		
N	BMVW-t	MT20	4.0	9.0		
O	BS-t	MT20	3.0	6.0		
P	BMVW-t	MT20	4.0	4.0	2.00	1.75
Q	BMVW-t	MT20	4.0	4.0		
R	BMVW-t	MT20	5.0	8.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
R	1906	0	1906	0	0	5-8	5-8
L	1906	0	1906	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
R	1548	886 / 0	335 / 0	0 / 0	0 / 0	327 / 0	0 / 0
L	1548	886 / 0	335 / 0	0 / 0	0 / 0	327 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.62 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MEMB.		FACTORED		MEMB.		FACTORED		MEMB.	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MAX. CSI (LC)	UNBRACED LENGTH	MEMB.	FORCE (LBS)	MAX. CSI (LC)	MEMB.	FORCE (LBS)	MAX. CSI (LC)	MEMB.	FORCE (LBS)
FR-TO		FROM	TO			FR-TO			FR-TO			FR-TO	
A-B	0 / 26	-84.3	-84.3	0.11 (1)	10.00	C-Q	0 / 192	0.04 (3)					
B-C	0 / 14	-84.3	-84.3	0.17 (1)	10.00	Q-D	0 / 295	0.07 (2)					
C-D	-2523 / 0	-84.3	-84.3	0.27 (1)	4.11	D-P	0 / 932	0.21 (1)					
D-E	-2943 / 0	-84.3	-84.3	0.51 (1)	3.62	P-E	-490 / 0	0.19 (1)					
E-F	-2942 / 0	-84.3	-84.3	0.50 (1)	3.62	E-N	-2 / 0	0.00 (1)					
F-G	-2942 / 0	-84.3	-84.3	0.50 (1)	3.62	N-G	-489 / 0	0.19 (1)					
G-H	-2941 / 0	-84.3	-84.3	0.50 (1)	3.63	H-N	0 / 930	0.21 (1)					
H-I	-2523 / 0	-84.3	-84.3	0.27 (1)	4.11	M-H	0 / 295	0.07 (2)					
I-J	0 / 14	-84.3	-84.3	0.17 (1)	10.00	M-I	0 / 192	0.04 (3)					
J-K	0 / 26	-84.3	-84.3	0.11 (1)	10.00	R-C	-2684 / 0	0.97 (1)					
R-B	-249 / 0	0.0	0.0	0.02 (1)	7.81	I-L	-2684 / 0	0.97 (1)					
L-J	-249 / 0	0.0	0.0	0.02 (1)	7.81								
R-Q	0 / 2181	-28.0	-28.0	0.62 (2)	10.00								
Q-P	0 / 2245	-28.0	-28.0	0.65 (2)	10.00								
P-O	0 / 2943	-28.0	-28.0	0.56 (1)	10.00								
O-N	0 / 2943	-28.0	-28.0	0.56 (1)	10.00								
N-M	0 / 2246	-28.0	-28.0	0.65 (2)	10.00								
M-L	0 / 2181	-28.0	-28.0	0.62 (2)	10.00								

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 3.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.08")

CALCULATED VERT. DEFL.(LL) = L/999 (0.20")

ALLOWABLE DEFL.(TL) = L/360 (1.08")

CALCULATED VERT. DEFL.(TL) = L/999 (0.33")

CSI: TC=0.51 (D-E:1), BC=0.65 (M-N:2), WB=0.97 (I-L:1), SS=0.21 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

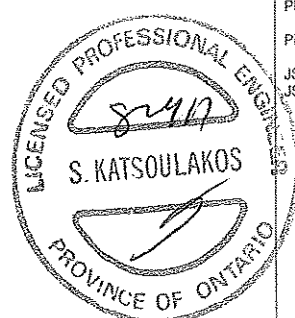
PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	622	2284	1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.87 (I) (INPUT = 0.90)

JSI METAL= 0.87 (O) (INPUT = 1.00)

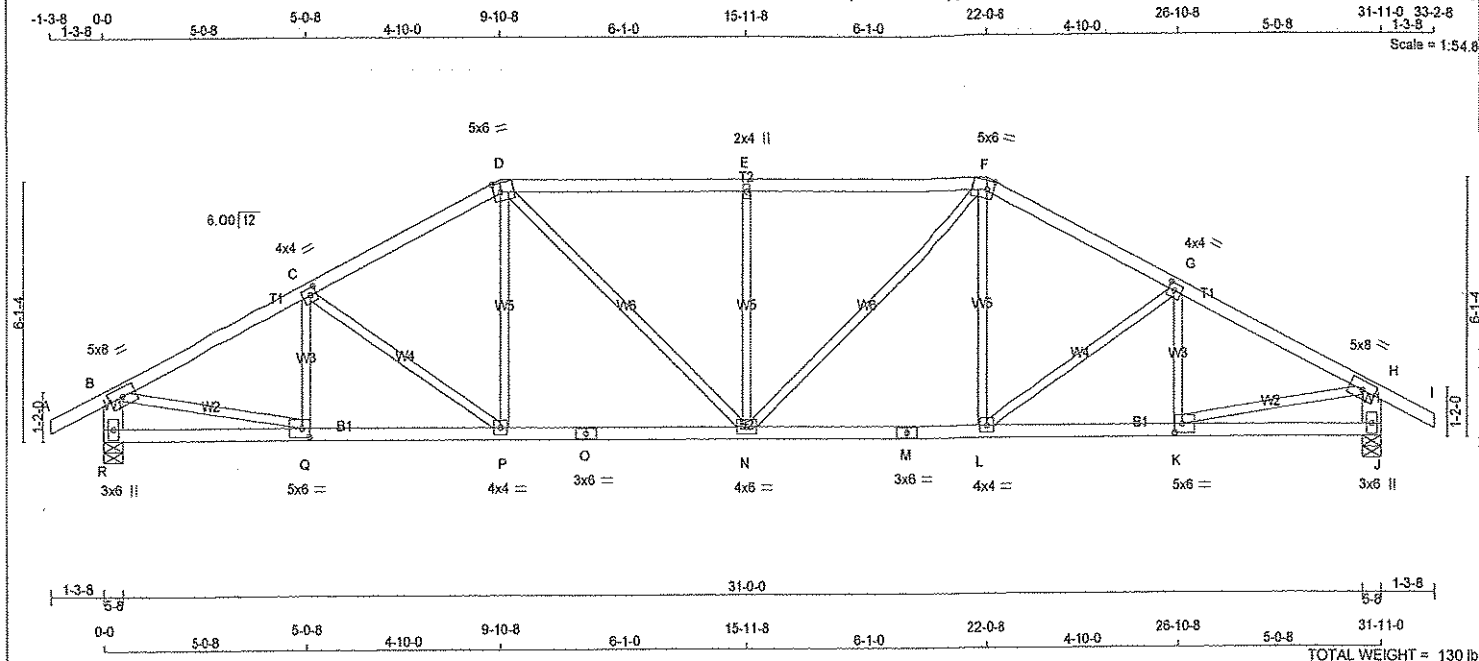


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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
282110	T3	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:3XqADRXd4x2pyhTJT2Ceyj0xP-pYVBczFdhE?g4ethw?44tvXbwnmMbxXJJIOjSylyFy



TOTAL WEIGHT = 130 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
D - F	2x4	DRY No.2	SPF
F - I	2x4	DRY No.2	SPF
R - B	2x6	DRY No.2	SPF
J - H	2x6	DRY No.2	SPF
R - O	2x4	DRY No.2	SPF
O - M	2x4	DRY No.2	SPF
M - J	2x4	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
B	TMVW-I	MT20	5.0	8.0
C	TMVW-I	MT20	4.0	4.0 2.00 1.75
D	TTWW-m	MT20	5.0	6.0 2.50 2.00
E	TMVW-w	MT20	2.0	4.0
F	TTWW-m	MT20	5.0	6.0 2.50 2.00
G	TMVW-I	MT20	4.0	4.0 2.00 1.75
H	TMVW-I	MT20	5.0	8.0
J	BMV1+p	MT20	3.0	6.0
K	BMVW-I	MT20	5.0	6.0 2.50 2.25
L	BMVW-I	MT20	4.0	4.0
M	BS-I	MT20	3.0	6.0
N	BMVW-I	MT20	4.0	6.0
O	BS-I	MT20	3.0	6.0
P	BMVW-I	MT20	4.0	4.0
Q	BMVW-I	MT20	5.0	6.0 2.50 2.25
R	BMV1+p	MT20	3.0	6.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
R	1906	0	1906	0
J	1906	0	1906	0

UNFACTORED REACTIONS

1ST LCASE	MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
R	1548	886 / 0	335 / 0	0 / 0	0 / 0	327 / 0
J	1548	886 / 0	335 / 0	0 / 0	0 / 0	327 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.85 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. MAX. CSI (LC) UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. MAX. CSI (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 26	-84.3 -84.3	0.11 (1)	10.00	Q-C	-255 / 68	0.08 (1)
B-C	-2585 / 0	-84.3 -84.3	0.35 (1)	4.01	C-P	-264 / 0	0.16 (1)
C-D	-2385 / 0	-84.3 -84.3	0.33 (1)	4.17	P-D	0 / 364	0.08 (2)
D-E	-2536 / 0	-84.3 -84.3	0.50 (1)	3.85	D-N	0 / 577	0.13 (1)
E-F	-2536 / 0	-84.3 -84.3	0.50 (1)	3.85	N-E	-628 / 0	0.37 (1)
F-G	-2385 / 0	-84.3 -84.3	0.33 (1)	4.17	N-F	0 / 577	0.13 (1)
G-H	-2585 / 0	-84.3 -84.3	0.35 (1)	4.01	L-F	0 / 364	0.08 (2)
H-I	0 / 26	-84.3 -84.3	0.11 (1)	10.00	L-G	-264 / 0	0.16 (1)
R-B	-1843 / 0	0.0 0.0	0.12 (1)	7.36	K-G	-255 / 68	0.08 (1)
J-H	-1843 / 0	0.0 0.0	0.12 (1)	7.36	B-Q	0 / 2364	0.53 (1)
R-Q	0 / 0	-28.0 -28.0	0.15 (2)	10.00	K-H	0 / 2364	0.53 (1)
Q-P	0 / 2331	-28.0 -28.0	0.47 (1)	10.00			
P-O	0 / 2119	-28.0 -28.0	0.46 (2)	10.00			
O-N	0 / 2119	-28.0 -28.0	0.46 (2)	10.00			
N-M	0 / 2119	-28.0 -28.0	0.46 (2)	10.00			
M-L	0 / 2119	-28.0 -28.0	0.46 (2)	10.00			
L-K	0 / 2331	-28.0 -28.0	0.47 (1)	10.00			
K-J	0 / 0	-28.0 -28.0	0.15 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 3.0 PSF
BOT CH. LL	= 10.5 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (1.06")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.16")
ALLOWABLE DEFL. (TL) = $L/360$ (1.06")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.25")

CSI: TC=0.50 (E-F:1), BC=0.47 (K-L:1), WB=0.53 (H-K:1), SSI=0.25 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

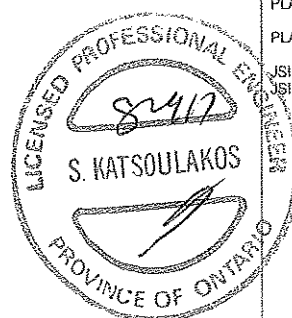
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

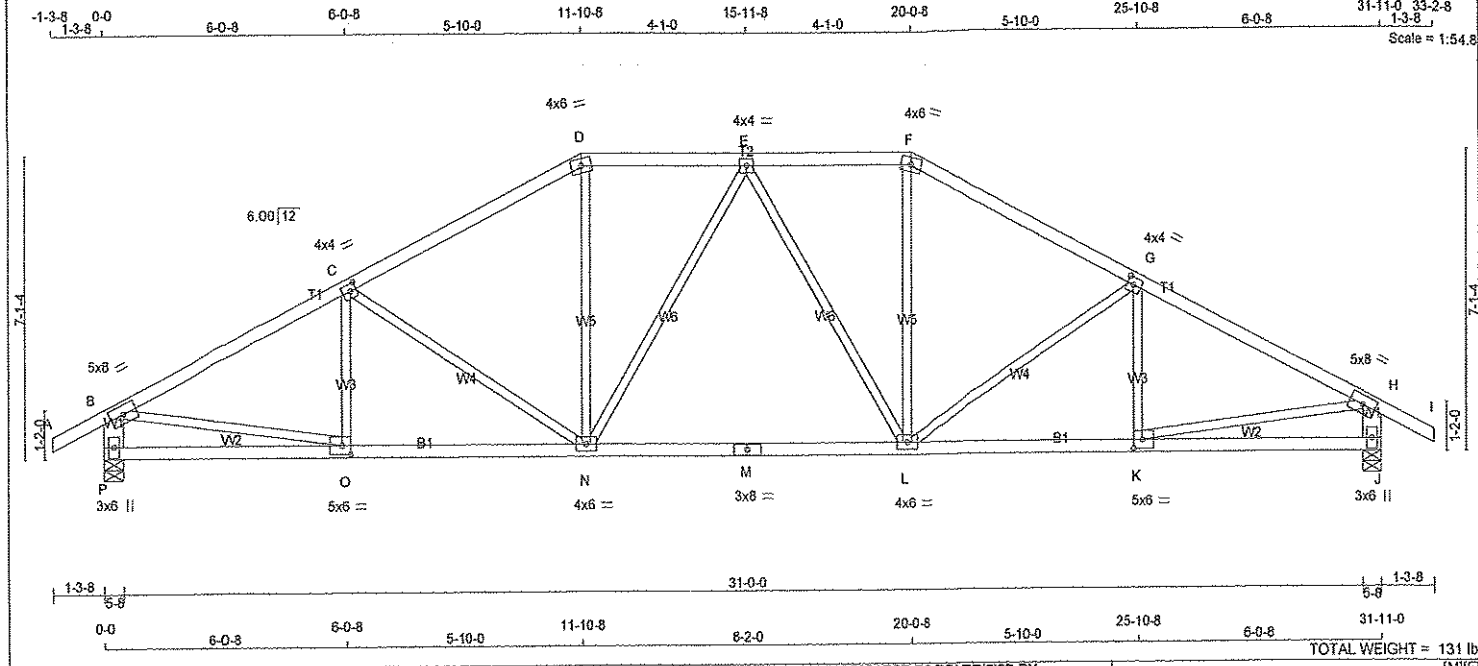
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

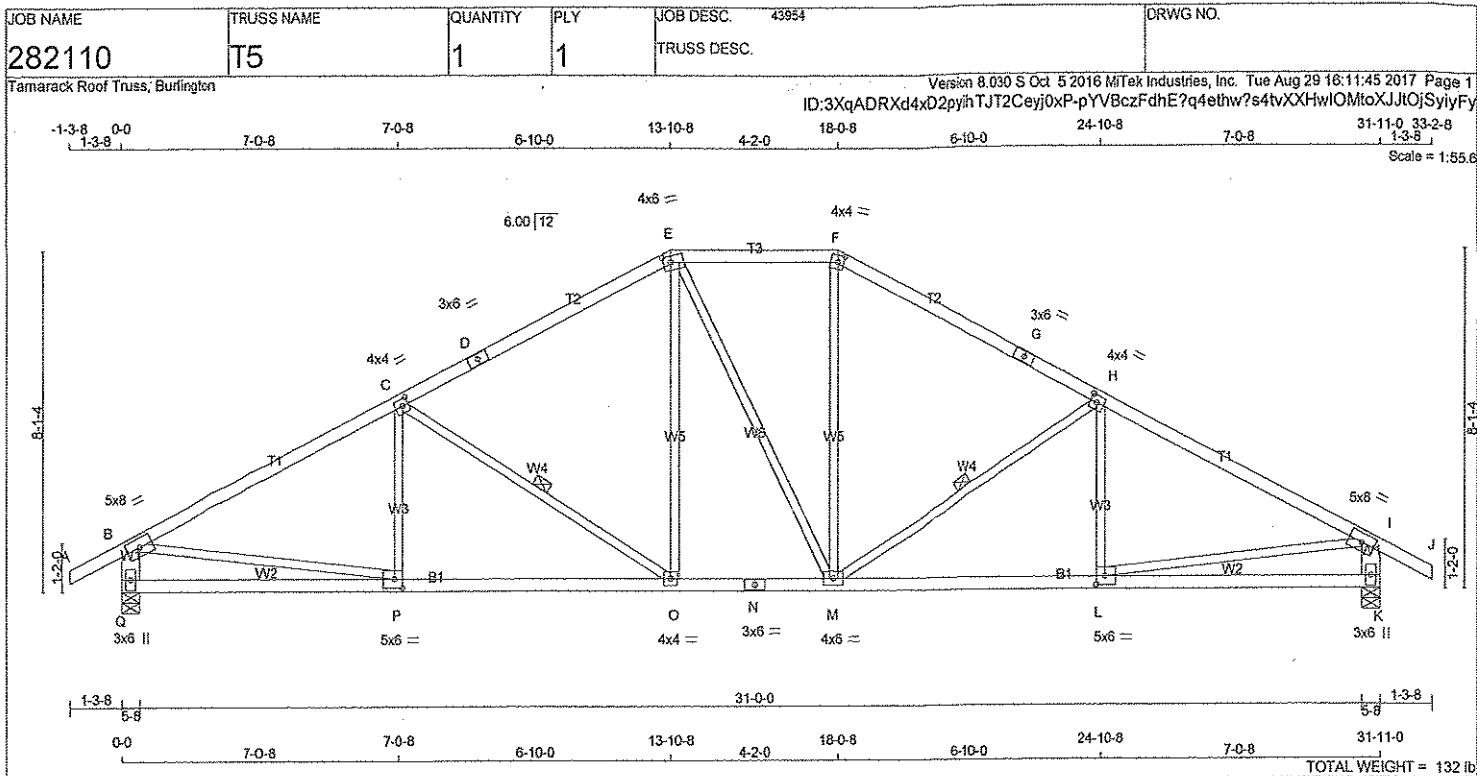
JSI GRIP = 0.87 (D) (INPUT = 0.90)
JSI METAL = 0.61 (O) (INPUT = 1.00)



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LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER										DESIGN CRITERIA			
N. L. G. A. RULES				BEARINGS										SPECIFIED LOADS:			
CHORDS	SIZE	LUMBER	DESCR.	FACTORED GROSS REACTION										TOP CH.	LL	=	25.6 PSF
A - D	2x4	DRY	No.2	MAXIMUM FACTORED GROSS REACTION										DL	=	3.0 PSF	
D - F	2x4	DRY	No.2	INPUT BRG										BOT CH.	LL	=	10.5 PSF
F - I	2x4	DRY	No.2	REQRD IN-SX										DL	=	7.0 PSF	
P - B	2x6	DRY	No.2	DOWN										TOTAL LOAD = 46.1 PSF			
J - H	2x6	DRY	No.2	UP										SPACING = 24.0 IN. C/C			
P - M	2x4	DRY	No.2	BRG										LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12			
M - J	2x4	DRY	No.2	IN-SX										THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010			
ALL WEBS	2x3	DRY	No.2	UNFACTORED REACTIONS										THIS DESIGN COMPLIES WITH:			
EXCEPT				1ST LCASE										- PART 9 OF OBC 2012, BCBC 2012, ABC 2014			
DRY: SEASONED LUMBER.				MAX/MIN. COMPONENT REACTIONS										- CSA 086-09			
				JT COMBINED										- TPIC 2011			
				P 1548										(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD			
				J 1906										ALLOWABLE DEFL. (LL) = L/360 (1.06")			
														CALCULATED VERT. DEFL. (LL) = L/999 (0.19")			
														ALLOWABLE DEFL. (TL) = L/360 (1.06")			
														CALCULATED VERT. DEFL. (TL) = L/999 (0.32")			
														CSI: TC=0.52 (G-H:1), BC=0.57 (K-L:2), WB=0.54 (H-K:1), SSI=0.22 (G-H:1)			
														DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10			
														COMP=1.10 SHEAR=1.10 TENS= 1.10			
														COMPANION LIVE LOAD FACTOR = 0.50			
														TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.			
														NAIL VALUES			
														PLATE GRIP(DRY) SHEAR SECTION			
														(PSI) (PLI) (PLI)			
														MAX MIN MAX MIN MAX MIN			
														MT20 618 354 1667 822 2284 1655			
														PLATE PLACEMENT TOL = 0.250 inches			
														PLATE ROTATION TOL = 5.0 Deg.			
														JSI GRIP= 0.90 (K) (INPUT = 0.90)			
														JSI METAL= 0.77 (M) (INPUT = 1.00)			
														DRWG. TAM 435417			
														STRUCTURAL			
														CONSENT ONLY			



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	DRY	No.2
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
G - J	2x4	DRY	No.2
Q - B	2x6	DRY	No.2
K - I	2x6	DRY	No.2
Q - N	2x4	DRY	No.2
N - K	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-I	MT20	5.0	8.0		
C	TMVW-I	MT20	4.0	4.0	2.00	1.75
D	TS-I	MT20	3.0	6.0		
E	TTWW-m	MT20	4.0	6.0	1.75	2.25
F	TTW-m	MT20	4.0	4.0	2.00	1.75
G	TS-I	MT20	3.0	6.0		
H	TMVW-I	MT20	4.0	4.0	2.00	1.75
I	TMVW-I	MT20	5.0	8.0		
K	BMV1+p	MT20	3.0	6.0		
L	BMVW-I	MT20	5.0	6.0	2.50	2.50
M	BMVW-I	MT20	4.0	6.0		
N	BS-I	MT20	3.0	6.0		
O	BMVW-I	MT20	4.0	4.0		
P	BMVW-I	MT20	5.0	6.0	2.50	2.50
Q	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
JT	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	IN-SX	IN-SX
Q	1906	0	1906	0	0	5-8	5-8	5-8	5-8
K	1906	0	1906	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN. COMPONENT REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL		
Q	1548	886 / 0	335 / 0	0 / 0	0 / 0	327 / 0	0 / 0		
K	1548	886 / 0	335 / 0	0 / 0	0 / 0	327 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.53 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-O, H-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX. FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 (LC)	MAX. UNBRACED	MEMB.	FORCE (LBS)	MAX. UNBRACED	MEMB.	FORCE (LBS)
FR-TO		FROM	TO	LENGTH	FR-TO				
A-B	0 / 26	-84.3	-84.3	0.11 (1)	10.00	P-C	-67 / 254	0.08 (3)	
B-C	-2636 / 0	-84.3	-84.3	0.74 (1)	3.53	C-O	-889 / 0	0.32 (1)	
C-D	-2048 / 0	-84.3	-84.3	0.63 (1)	4.03	O-E	0 / 509	0.11 (1)	
D-E	-2048 / 0	-84.3	-84.3	0.63 (1)	4.03	E-M	0 / 2	0.00 (1)	
E-F	-1810 / 0	-84.3	-84.3	0.23 (1)	4.77	M-F	0 / 512	0.12 (1)	
F-G	-2049 / 0	-84.3	-84.3	0.63 (1)	4.03	M-H	-588 / 0	0.31 (1)	
G-H	-2049 / 0	-84.3	-84.3	0.63 (1)	4.03	H-H	-69 / 253	0.06 (3)	
H-I	-2636 / 0	-84.3	-84.3	0.74 (1)	3.53	B-P	0 / 2403	0.54 (1)	
I-J	0 / 26	-84.3	-84.3	0.11 (1)	10.00	L-I	0 / 2403	0.54 (1)	
Q-B	-1825 / 0	0.0	0.0	0.12 (1)	7.39				
K-I	-1825 / 0	0.0	0.0	0.12 (1)	7.39				
Q-P	0 / 0	-28.0	-28.0	0.35 (3)	10.00				
P-O	0 / 2386	-28.0	-28.0	0.62 (2)	10.00				
O-N	0 / 1809	-28.0	-28.0	0.39 (1)	10.00				
N-M	0 / 1809	-28.0	-28.0	0.39 (1)	10.00				
M-L	0 / 2385	-28.0	-28.0	0.62 (2)	10.00				
L-K	0 / 0	-28.0	-28.0	0.35 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (1.06")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.16")
ALLOWABLE DEFL.(TL) = $L/360$ (1.06")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.27")

CSI: TC=0.74 (B-C:1), BC=0.62 (O-P:2), WB=0.54 (B-P:1), SSI=0.26 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

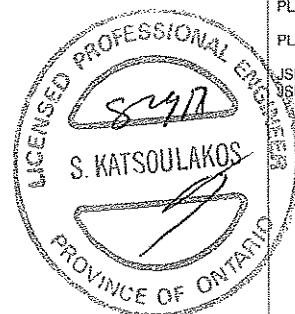
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

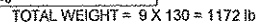
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (P) (INPUT = 0.90)
JSI METAL= 0.60 (P) (INPUT = 1.00)

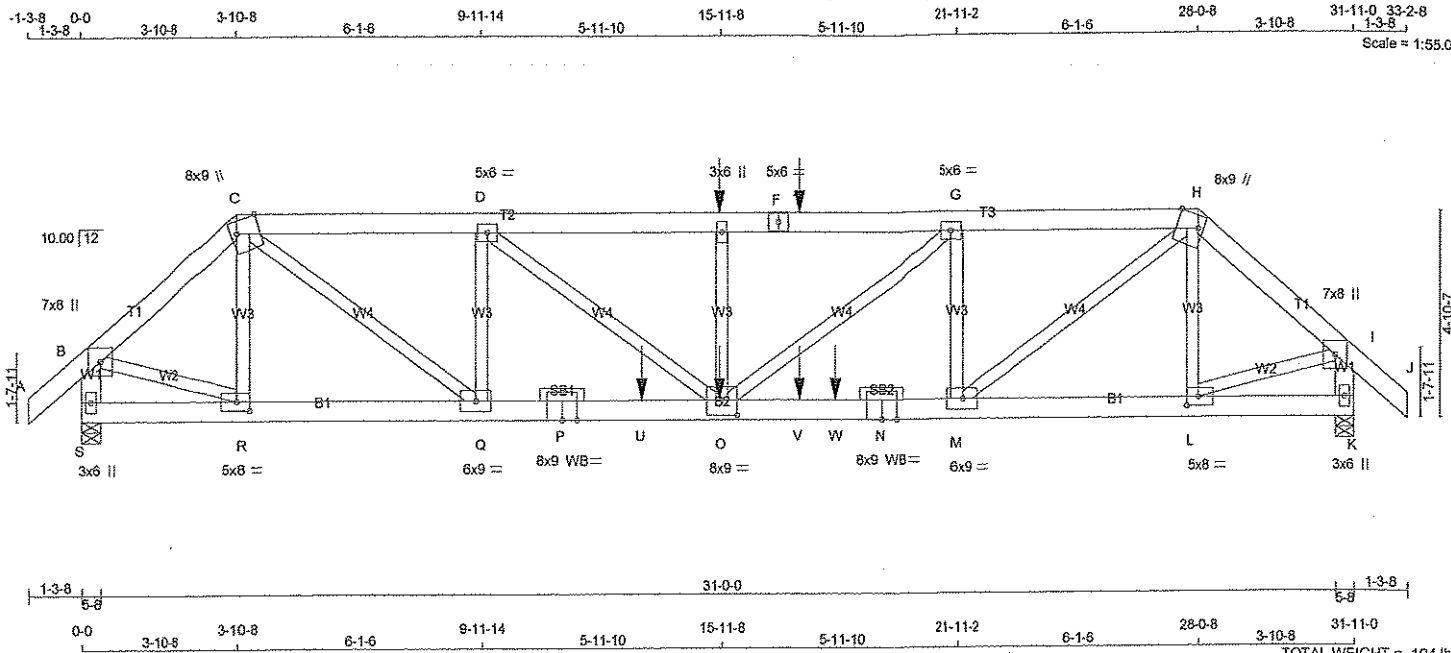


DRWG NO. TAM 4352-17
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Tamarack Roof Truss, Burlington

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ID:3XqADRXd4x02pyihTJT2Ceyj0xP-Ik3ZpJGFSX7hioSuUjNJK63lkK2Y5lvhYzdxFuyiyFx



TOTAL WEIGHT = 194 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
A - C	2x6	DRY	No.2 SPF
C - F	2x6	DRY	No.2 SPF
F - H	2x6	DRY	No.2 SPF
H - J	2x6	DRY	No.2 SPF
S - B	2x6	DRY	No.2 SPF
K - I	2x6	DRY	No.2 SPF
S - P	2x6	DRY	2100F 1.8E SPF
P - N	2x6	DRY	2100F 1.8E SPF
N - K	2x6	DRY	2100F 1.8E SPF

ALL WEBS 2x4 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVV+p	MT20	7.0	8.0		
C	TTVVW+m	MT20	8.0	9.0	Edge 6.50	
D	TMWW-t	MT20	5.0	6.0		
E	TMWW-w	MT20	3.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMWW-l	MT20	5.0	6.0		
H	TTVVW+m	MT20	8.0	9.0	Edge 6.50	
I	TMVV+p	MT20	7.0	8.0		
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	8.0	2.50	3.50
M	BMWW-l	MT20	6.0	9.0		
N	BS-t	MT20	8.0	9.0		
O	BMWWW-t	MT20	8.0	9.0	4.25	4.50
P	BS-t	MT20	8.0	9.0		
Q	BMWW-t	MT20	6.0	9.0		
R	BMWW-l	MT20	5.0	8.0	2.50	3.50
S	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

WB - INDICATES BLOCKING REQUIRED

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 38.3 lbs FACTORED DOWN AT 15-10-12, AND 38.3 lbs FACTORED DOWN AT 17-10-12 ON TOP CHORD, AND 888.3 lbs FACTORED DOWN AT 13-11-8, 34.1 lbs FACTORED DOWN AT 15-10-12, AND 34.1 lbs FACTORED DOWN AT 17-10-12, AND 1153.4 lbs FACTORED DOWN AT 18-9-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQD
JT	VERT	HORZ	DOWN	HORZ
S	2941	0	2941	0
K	3041	0	3041	0

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
S	2363	1398 / 0	482 / 0	0 / 0	0 / 0	485 / 0
K	2442	1445 / 0	497 / 0	0 / 0	0 / 0	501 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.96 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1) (PLF)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC1) (PLF)	MAX. FACTORED VERT. LOAD (LC1) (PLF)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 39	-84.3	-84.3 0.07 (1)	10.00	R-C	-512 / 0	0.12 (1)
B-C	-3055 / 0	-84.3	-84.3 0.17 (1)	4.66	C-Q	0 / 3563	0.63 (1)
C-D	-5216 / 0	-84.3	-84.3 0.42 (1)	3.50	Q-D	-1718 / 0	0.39 (1)
D-E	-5798 / 0	-84.3	-84.3 0.58 (1)	2.97	D-O	0 / 1965	0.35 (1)
E-F	-5798 / 0	-84.3	-84.3 0.58 (1)	2.98	O-E	-516 / 0	0.12 (1)
F-T	-6798 / 0	-84.3	-84.3 0.58 (1)	2.98	O-G	0 / 1612	0.29 (1)
T-G	-6798 / 0	-84.3	-84.3 0.58 (1)	2.98	M-G	-1520 / 0	0.34 (1)
G-H	-5501 / 0	-84.3	-84.3 0.45 (1)	3.39	M-H	0 / 3825	0.68 (1)
H-I	-3152 / 0	-84.3	-84.3 0.18 (1)	4.60	L-H	-603 / 0	0.14 (1)
I-J	0 / 39	-84.3	-84.3 0.07 (1)	10.00	B-R	0 / 2424	0.43 (1)
S-B	-2699 / 0	0.0	0.0 0.21 (1)	6.12	L-I	0 / 2502	0.44 (1)
K-I	-2983 / 0	0.0	0.0 0.21 (1)	6.05			

S-R	0 / 0	-28.0	-28.0 0.05 (3)	10.00
R-Q	0 / 2319	-28.0	-28.0 0.19 (1)	10.00
Q-P	0 / 5217	-28.0	-28.0 0.65 (1)	10.00
P-U	0 / 5217	-28.0	-28.0 0.65 (1)	10.00
U-O	0 / 5217	-28.0	-28.0 0.65 (1)	10.00
O-V	0 / 5501	-28.0	-28.0 0.76 (1)	10.00
V-W	0 / 5501	-28.0	-28.0 0.76 (1)	10.00
W-N	0 / 5501	-28.0	-28.0 0.76 (1)	10.00
N-M	0 / 5501	-28.0	-28.0 0.76 (1)	10.00
M-L	0 / 2391	-28.0	-28.0 0.29 (1)	10.00
L-K	0 / 0	-28.0	-28.0 0.03 (3)	10.00

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.
E	15-10-12	-38	-38	-	BACK	VERT
O	15-10-12	-20	-34	-	BACK	VERT
T	17-10-12	-38	-38	-	BACK	VERT
U	13-11-8	-888	-888	-	BACK	VERT
V	17-10-12	-20	-34	-	BACK	VERT
W	18-9-8	-1153	-1153	-	BACK	VERT

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (1.06")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.30")
ALLOWABLE DEFL. (TL) = $L/360$ (1.06")
CALCULATED VERT. DEFL. (TL) = $L/806$ (0.47")

CSI: TC=0.56 (E-G:1), BC=0.76 (M-O:1),
WB=0.68 (H-M:1), SSI=0.53 (O-Q:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

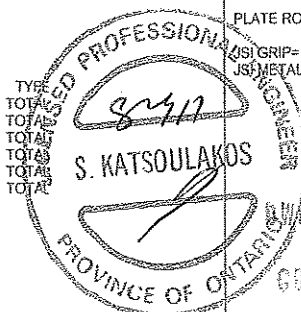
NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
MT20	618	354
	1667	822
	2284	1658

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

USE GRIP= 0.90 (N) (INPUT = 0.90)
USE METAL= 0.88 (P) (INPUT = 1.00)



43524
STRUCTURAL
CONSENT ONLY

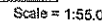
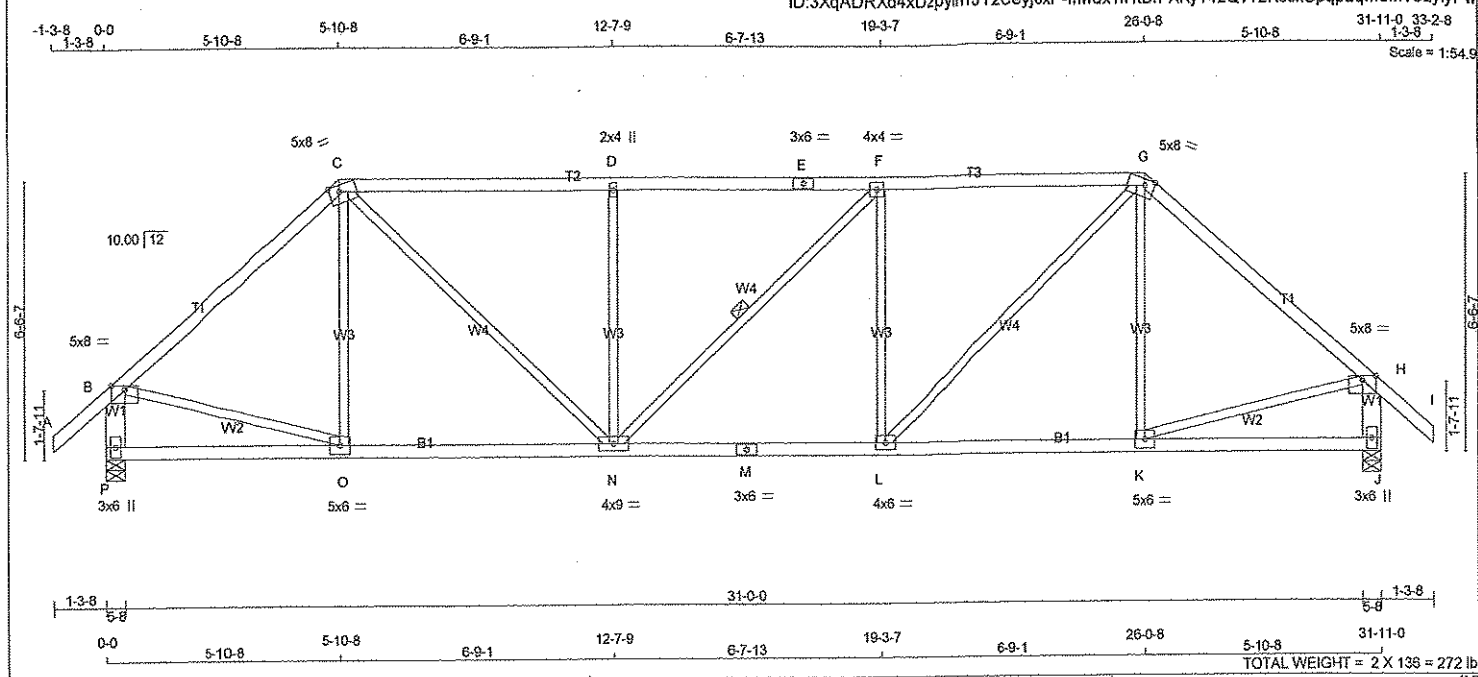


PLATE ROTATION TOL. = 5.0 Deg.

STRUCTURAL
COMPONENT ONLY



<u>LUMBER</u>				
N. L. G. A. RULES				
CHORDS	SIZE		LUMBER	DESCR
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
J - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
DRY- SEASONED LUMBER				

<u>PLATES (table is in inches)</u>			
JT	TYPE	PLATES	W LEN Y X
B	TMWV-p	MT20	5.0 8.0 Edge
C	TTWW-m	MT20	5.0 8.0 Edge 3.00
D	TMW+w	MT20	2.0 4.0
E	TS-t	MT20	3.0 6.0
F	TMWW-t	MT20	4.0 4.0
G	TTWW-m	MT20	5.0 8.0 Edge 3.00
H	TMWV-p	MT20	5.0 8.0 Edge
J	BMV1+p	MT20	3.0 6.0
K	BMWWW-t	MT20	5.0 6.0
L	BMWW-t	MT20	4.0 6.0
M	BS-t	MT20	3.0 6.0
N	BMWWW-t	MT20	4.0 9.0
O	BMWW-t	MT20	5.0 6.0
P	BMV1+p	MT20	3.0 6.0

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER						
<u>BEARINGS</u>						
	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REORD BRG
JT	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX
P	1908	0	1908	0	5-8	5-8
J	1908	0	1908	0	5-8	5-8

UNFACTORED REACTIONS							
JT	1ST CASE	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
P	1550	887 / 0	335 / 0	0 / 0	0 / 0	327 / 0	0 / 0
J	1550	887 / 0	335 / 0	0 / 0	0 / 0	327 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING									
TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)		
FR-TO		FROM TO			FR-TO				
A-B	0 / 37	-84.3	-84.3	0.12 (1)	10.00	O-C	-107 / 188	0.07 (1)	
B-C	-1828 / 0	-84.3	-84.3	0.71 (1)	4.19	C-N	0 / 1173	0.28 (1)	
C-D	-2262 / 0	-84.3	-84.3	0.73 (1)	3.78	N-D	-611 / 0	0.43 (1)	
D-E	-2262 / 0	-84.3	-84.3	0.73 (1)	3.77	N-F	-2 / 0	0.00 (1)	
E-F	-2262 / 0	-84.3	-84.3	0.73 (1)	3.77	L-F	-612 / 0	0.43 (1)	
F-G	-2263 / 0	-84.3	-84.3	0.74 (1)	3.77	L-G	0 / 1175	0.28 (1)	
G-H	-1827 / 0	-84.3	-84.3	0.71 (1)	4.19	K-G	-108 / 188	0.08 (1)	
H-I	0 / 37	-84.3	-84.3	0.12 (1)	10.00	B-O	0 / 1434	0.32 (1)	
P-B	-1843 / 0	0.0	0.0	0.13 (1)	7.36	K-H	0 / 1434	0.32 (1)	
J-H	-1843 / 0	0.0	0.0	0.13 (1)	7.36				
P-O	0 / 0	-28.0	-28.0	0.27 (3)	10.00				
O-N	0 / 1400	-28.0	-28.0	0.44 (2)	10.00				
N-M	0 / 2264	-28.0	-28.0	0.50 (2)	10.00				
M-L	0 / 2264	-28.0	-28.0	0.50 (2)	10.00				
L-K	0 / 1399	-28.0	-28.0	0.44 (2)	10.00				
K-J	0 / 0	-28.0	-28.0	0.27 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= $L/360$ (1.06")
 CALCULATED VERT. DEFL.(LL) = $L/999$ (0.13")
 ALLOWABLE DEFL.(TL)= $L/360$ (1.06")
 CALCULATED VERT. DEFL.(TL) = $L/999$ (0.22")

CSI: TC=0.74 (F-G:1), BC=0.50 (L-N:2), WB=0.43 (F-L:1), SSI=0.26 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
(IN)	(KSI)	(KSI)	(IN)

	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.86 (G) (INPUT = 0.90)
JSI METAL= 0.71 (M) (INPUT = 1.00)



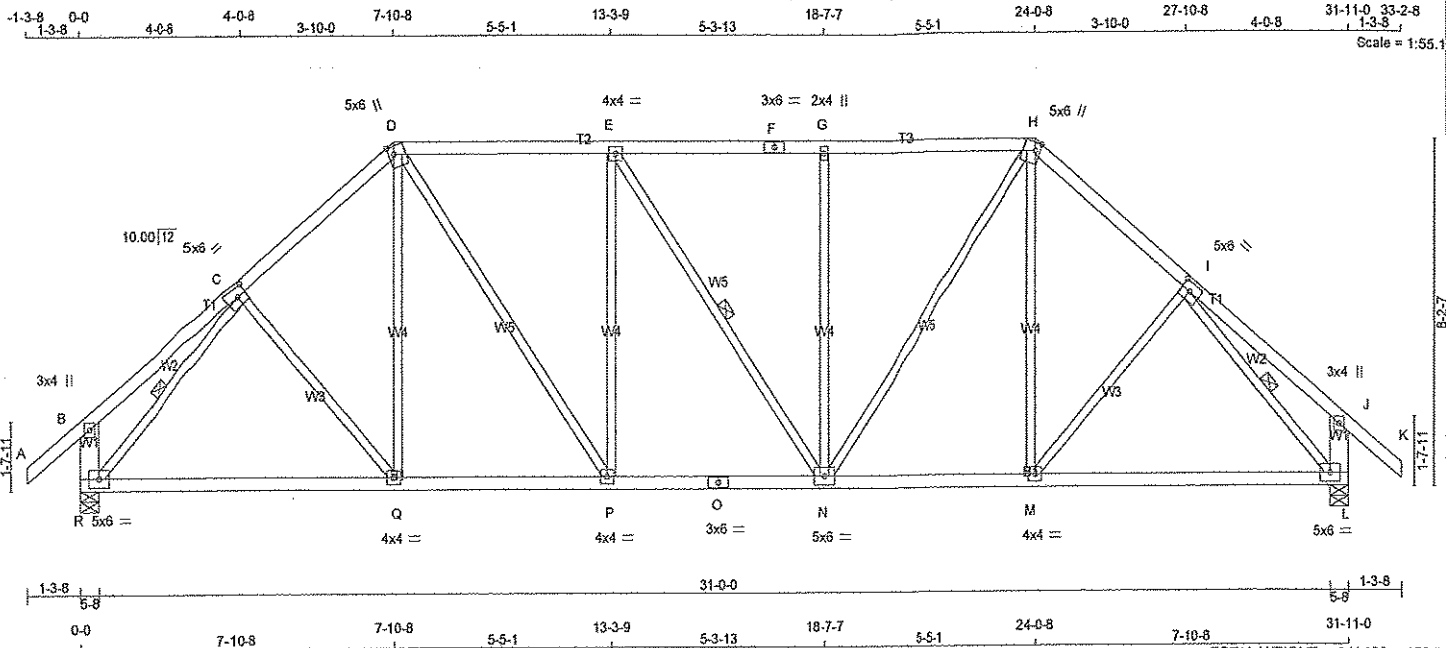
DIVISION 43525-17
 STRUCTURAL
 COMPONENT ONLY

JOB NAME 282110	TRUSS NAME T10	QUANTITY 3	PLY 1	JOB DESC. 43954 TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:3XqADRXd4x2D2pyihTJT2CeyJ0xP-mwdx1fHtDrFXKy142QvYzKoyvKR1qmkqmdMVoLylyFw



TOTAL WEIGHT = 3 X 153 = 459 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
R - B	2x6	DRY	No.2
L - J	2x6	DRY	No.2
R - O	2x4	DRY	No.2
O - L	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
C	TMVW+1	MT20	5.0	6.0	2.50 2.75
D	TTWW+m	MT20	5.0	6.0	2.25 1.50
E	TMVW+1	MT20	4.0	4.0	
F	TS-1	MT20	3.0	6.0	
G	TMVW+w	MT20	2.0	4.0	
H	TTWW+m	MT20	5.0	6.0	2.25 1.50
I	TMVW+1	MT20	5.0	6.0	2.50 2.75
J	TMV+p	MT20	3.0	4.0	
L	BMVW1-1	MT20	5.0	6.0	
M, P, Q					
M	BMVW+1	MT20	4.0	4.0	
N	BMVW+1	MT20	5.0	6.0	
O	BS-1	MT20	3.0	6.0	
R	BMVW1-1	MT20	5.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	BRG	BRG
R	1908	0	1908	0	0	5-8	5-8		
L	1908	0	1908	0	0	5-8	5-8		

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
R	1550	887 / 0	335 / 0	0 / 0	0 / 0	327 / 0	0 / 0		
L	1550	887 / 0	335 / 0	0 / 0	0 / 0	327 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.55 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N, C-R, I-L

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO				FR-TO			
A-B	0 / 37	-84.3	-84.3 0.12 (1)	10.00	C-Q	0 / 133	0.03 (3)
B-C	0 / 23	-84.3	-84.3 0.20 (1)	10.00	Q-D	0 / 311	0.07 (2)
C-D	-1805 / 0	-84.3	-84.3 0.26 (1)	4.73	D-P	0 / 748	0.17 (1)
D-E	-1792 / 0	-84.3	-84.3 0.41 (1)	4.55	P-E	-488 / 0	0.64 (1)
E-F	-1791 / 0	-84.3	-84.3 0.41 (1)	4.55	E-N	-2 / 0	0.00 (1)
F-G	-1791 / 0	-84.3	-84.3 0.41 (1)	4.55	N-G	-487 / 0	0.64 (1)
G-H	-1791 / 0	-84.3	-84.3 0.40 (1)	4.57	H-H	0 / 748	0.17 (1)
H-I	-1805 / 0	-84.3	-84.3 0.26 (1)	4.73	M-H	0 / 312	0.07 (2)
I-J	0 / 23	-84.3	-84.3 0.20 (1)	10.00	M-I	0 / 133	0.03 (3)
J-K	0 / 37	-84.3	-84.3 0.12 (1)	10.00	R-C	-2069 / 0	0.51 (1)
R-B	-245 / 0	0.0	0.0 0.02 (1)	7.81	I-L	-2069 / 0	0.51 (1)
L-J	-245 / 0	0.0	0.0 0.02 (1)	7.81			
R-Q	0 / 1346	-28.0	-28.0 0.53 (2)	10.00			
Q-P	0 / 1368	-28.0	-28.0 0.55 (2)	10.00			
P-O	0 / 1792	-28.0	-28.0 0.37 (1)	10.00			
O-N	0 / 1792	-28.0	-28.0 0.37 (1)	10.00			
N-M	0 / 1368	-28.0	-28.0 0.55 (2)	10.00			
M-L	0 / 1346	-28.0	-28.0 0.53 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	25.6	PSF
DL	=	3.9	PSF
BOT CH. LL	=	10.5	PSF
DL	=	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/380 (1.06")
CALCULATED VERT. DEFL.(LL)= L/999 (0.17")
ALLOWABLE DEFL.(TL)= L/380 (1.06")
CALCULATED VERT. DEFL.(TL)= L/999 (0.29")

CSI: TC=0.41 (D-E:1), BC=0.55 (M-N:2), WB=0.64 (E-P:1), SSI=0.21 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

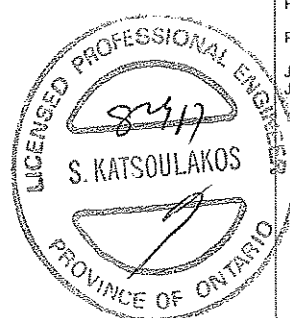
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

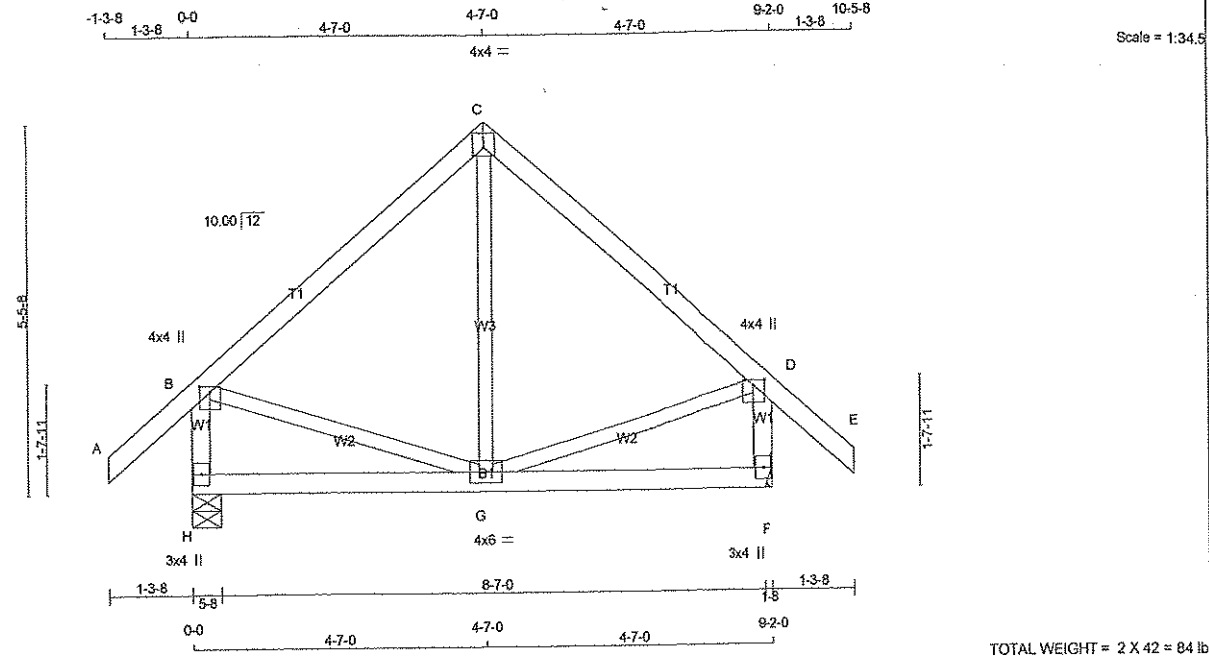
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.88 (I) (INPUT = 0.90)
JSI METAL = 0.58 (O) (INPUT = 1.00)



DRGNO. TAM 4352617
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
H - B	2x4	DRY	No.2
F - D	2x4	DRY	No.2
H - F	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW-p	MT20	4.0	4.0	1.50	2.00
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMVWW-l	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
H	631	0	631	0	5-8	5-8
F	631	0	631	0	5-8	5-8

HANGER BY OTHERS
MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
H	502	305 / 0	96 / 0	0 / 0	0 / 0	100 / 0	0 / 0
F	502	305 / 0	96 / 0	0 / 0	0 / 0	100 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LG1 MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED LG1 MAX. CSI (LC)	MEMB.
FR-TO				FR-TO			
A-B	0 / 37	-84.3	-84.3 0.12 (1)	10.00	G-C	0 / 199	0.05 (3)
B-C	-317 / 0	-84.3	-84.3 0.23 (1)	6.25	B-G	0 / 253	0.06 (1)
C-D	-317 / 0	-84.3	-84.3 0.23 (1)	6.25	G-D	0 / 253	0.06 (1)
D-E	0 / 37	-84.3	-84.3 0.12 (1)	10.00			
H-B	-582 / 0	0.0	0.0 0.06 (1)	7.81			
F-D	-582 / 0	0.0	0.0 0.06 (1)	7.81			
H-G	0 / 0	-28.0	-28.0 0.17 (3)	10.00			
G-F	0 / 0	-28.0	-28.0 0.17 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	PSF
	25.6	PSF
BOT CH.	DL	PSF
	3.0	PSF
	10.5	PSF
	7.0	PSF
TOTAL LOAD	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.31")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.31")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.23 (C-D:1), BC=0.17 (G-H:3), WB=0.06 (B-G:1), SSI=0.12 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

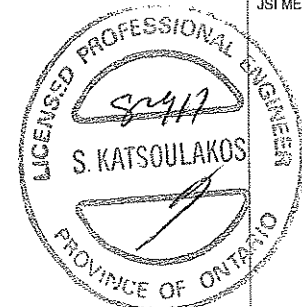
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	822	2284	1658

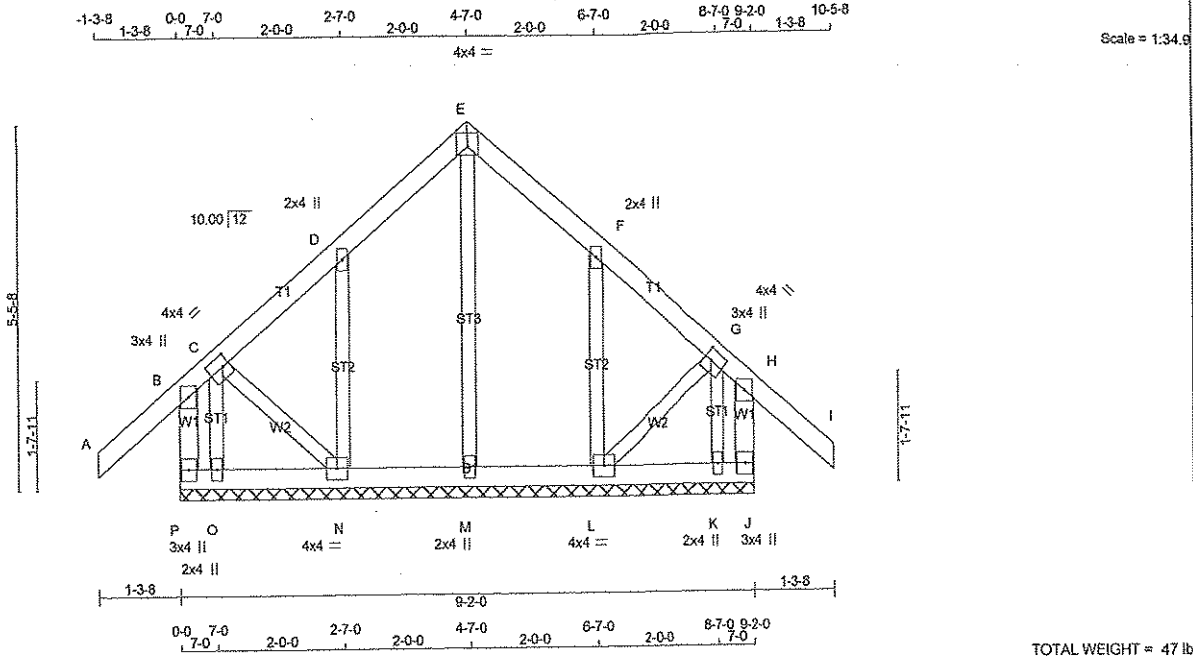
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.67 (C) (INPUT = 0.90)
JSI METAL = 0.14 (B) (INPUT = 1.00)



5W6R0.YAM4352) -17
STRUCTURAL
COMPONENT ONLY



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE		LUMBER
A - E	2x4	DRY	No.2
E - I	2x4	DRY	No.2
P - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
P - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			
GABLE STUDS SPACED AT 2-0-0 OC.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 3.0 PSF

BOT CH. LL = 10.5 PSF
DL = 7.0 PSF

TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	3.0	4.0
C	TMWW-l	MT20	4.0	4.0 2.00 1.25
D	TMW+w	MT20	2.0	4.0
E	TTW-p	MT20	4.0	4.0 1.50 2.00
F	TMW+w	MT20	2.0	4.0
G	TMWW-l	MT20	4.0	4.0 2.00 1.25
H	TMV+p	MT20	3.0	4.0
J	BMV1+p	MT20	3.0	4.0
K, M, O				
K	BMW1+w	MT20	2.0	4.0
L	BMWW1-l	MT20	4.0	4.0
N	BMWW1-l	MT20	4.0	4.0
P	BMV1+p	MT20	3.0	4.0

LOADING									
TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX. (PLF)	LC2 MAX. (PLF)	MEMB.	MAX. FACTORED FORCE (LBS)	LC1 MAX. (PLF)	LC2 MAX. (PLF)	UNBRACED LENGTH (FT)
FR-TO		FROM	TO		FR-TO		FROM	TO	
A-B	0/37	-84.3	-84.3	0.12 (1)	10.00	M-E	-101/0	0.04 (1)	
B-C	-75/0	-84.3	-84.3	0.11 (1)	6.25	N-D	-195/0	0.04 (1)	
C-D	-30/0	-84.3	-84.3	0.05 (1)	6.25	O-C	-26/0	0.00 (1)	
D-E	-42/0	-84.3	-84.3	0.05 (1)	6.25	L-F	-195/0	0.04 (1)	
E-F	-42/0	-84.3	-84.3	0.05 (1)	6.25	K-G	-26/0	0.00 (1)	
F-G	-30/0	-84.3	-84.3	0.05 (1)	6.25	C-N	0/39	0.01 (1)	
G-H	-75/0	-84.3	-84.3	0.11 (1)	6.25	L-G	0/39	0.01 (1)	
H-I	0/37	-84.3	-84.3	0.12 (1)	10.00				
P-B	-258/0	0.0	0.0	0.03 (1)	7.81				
J-H	-258/0	0.0	0.0	0.03 (1)	7.81				
P-O	0/0	-28.0	-28.0	0.02 (3)	10.00				
O-N	0/0	-28.0	-28.0	0.02 (3)	10.00				
N-M	0/24	-28.0	-28.0	0.03 (2)	10.00				
M-L	0/24	-28.0	-28.0	0.03 (2)	10.00				
L-K	0/0	-28.0	-28.0	0.02 (3)	10.00				
K-J	0/0	-28.0	-28.0	0.02 (3)	10.00				

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

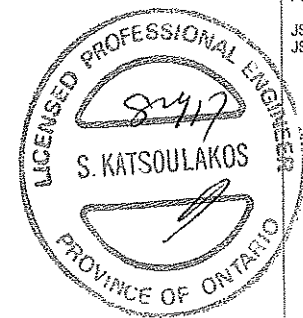
(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.12 (A-B:1), BC=0.03 (M-N:2), WB=0.04 (D-N:1), SSI=0.08 (G-H:1)

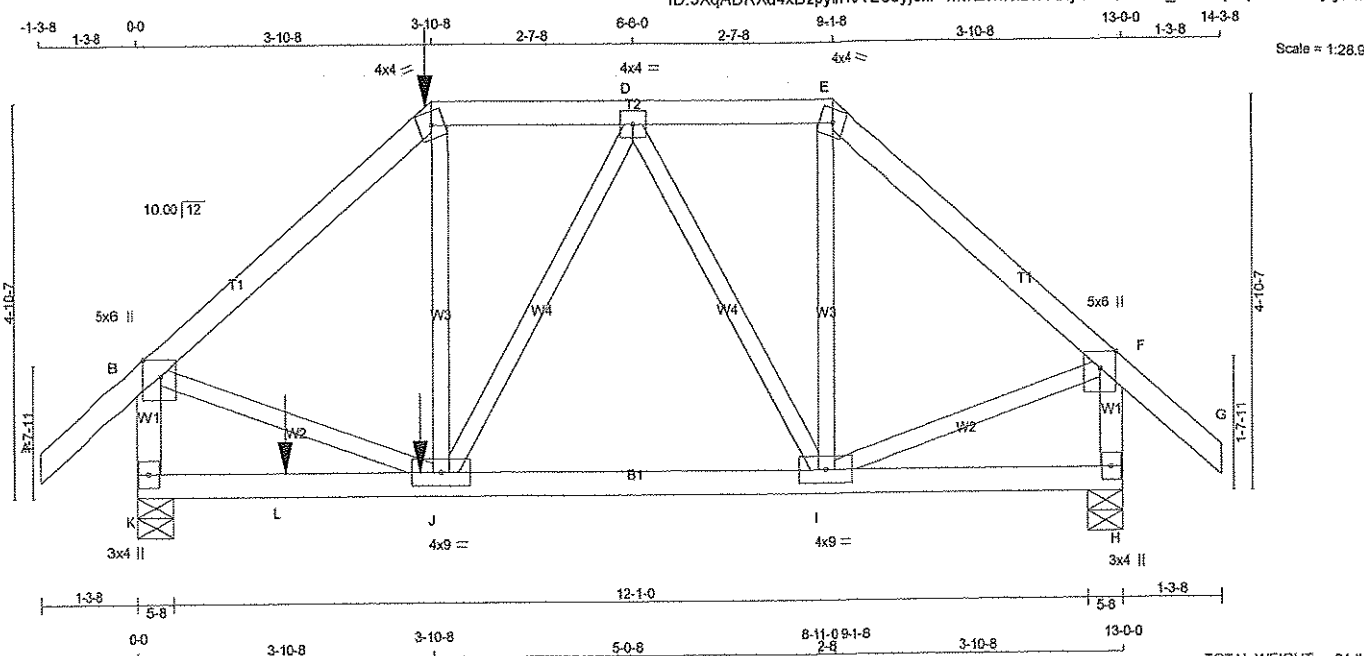
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.



DRWG NO. TAM 4357517
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 61 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	No.2	LUMBER	DESCR	SPF
A - C	2x4	DRY	No.2		SPF	
C - E	2x4	DRY	No.2		SPF	
E - G	2x4	DRY	No.2		SPF	
K - B	2x4	DRY	No.2		SPF	
H - F	2x4	DRY	No.2		SPF	
K - H	2x4	DRY	No.2		SPF	

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	5.0	6.0	Edge	
C TTW-m	MT20	4.0	4.0		
D TMWW-t	MT20	4.0	4.0		
E TTW-m	MT20	4.0	4.0		
F TMVW+p	MT20	5.0	6.0	Edge	
H BMV1+p	MT20	3.0	4.0		
I BMWWW-t	MT20	4.0	9.0		
J BMWWW-t	MT20	4.0	9.0		
K BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 138.5 lbs FACTORED DOWN AT 3-10-8 ON TOP CHORD, AND 42.0 lbs FACTORED DOWN AT 1-11-4, AND 797.4 lbs FACTORED DOWN AT 3-8-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
K	1517	0	1517	0	5-8	5-8
H	1135	0	1135	0	5-8	5-8

UNFACTORED REACTIONS

JT	COMBINED	MAX./MIN. COMPONENT REACTIONS				DEAD	SOIL
		SNOW	LIVE	PERM. LIVE	WIND		
K	1180	781 / 0	198 / 0	0 / 0	0 / 0	221 / 0	0 / 0
H	892	560 / 0	159 / 0	0 / 0	0 / 0	172 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.27 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
									FR-TO
A-B	0 / 37	-84.3	-84.3	0.13 (1)	10.00	J-C	0 / 451	0.11 (1)	
B-C	-1323 / 0	-84.3	-84.3	0.28 (1)	5.27	I-E	0 / 327	0.08 (1)	
C-D	-1023 / 0	-84.3	-84.3	0.10 (1)	6.06	B-J	0 / 1068	0.28 (1)	
D-E	-704 / 0	-84.3	-84.3	0.09 (1)	6.25	I-F	0 / 733	0.18 (1)	
E-F	-909 / 0	-84.3	-84.3	0.26 (1)	6.10	J-D	0 / 188	0.05 (1)	
F-G	0 / 37	-84.3	-84.3	0.13 (1)	10.00	D-I	-474 / 0	0.22 (1)	
K-B	-1482 / 0	0.0	0.0	0.17 (1)	6.73				
H-F	-1093 / 0	0.0	0.0	0.13 (1)	7.53				
K-L	0 / 0	-28.0	-28.0	0.17 (3)	10.00				
L-J	0 / 0	-28.0	-28.0	0.17 (3)	10.00				
J-I	0 / 933	-28.0	-28.0	0.26 (2)	10.00				
I-H	0 / 0	-28.0	-28.0	0.14 (3)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	3-10-8	-13	-14	---	FRONT	VERT	DEAD
C	3-10-8	-126	-126	---	FRONT	VERT	SNOW
J	3-8-8	-797	-797	---	FRONT	VERT	TOTAL
L	1-11-4	-24	-42	---	FRONT	VERT	TOTAL



DESIGN CRITERIA
*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/380 (0.43")
CALCULATED VERT. DEFL. (LL) = L/999 (0.02")
ALLOWABLE DEFL. (TL) = L/360 (0.43")
CALCULATED VERT. DEFL. (TL) = L/999 (0.04")

CSI: TC=0.28 (B-C:1), BC=0.26 (I-J:2), WB=0.26 (B-J:1), SSI=0.13 (J-K:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (B) (INPUT = 0.90)
JSI METAL = 0.25 (F) (INPUT = 1.00)

TOTAL WEIGHT = 61 lb

STRUCTURAL
COMPONENT ONLY



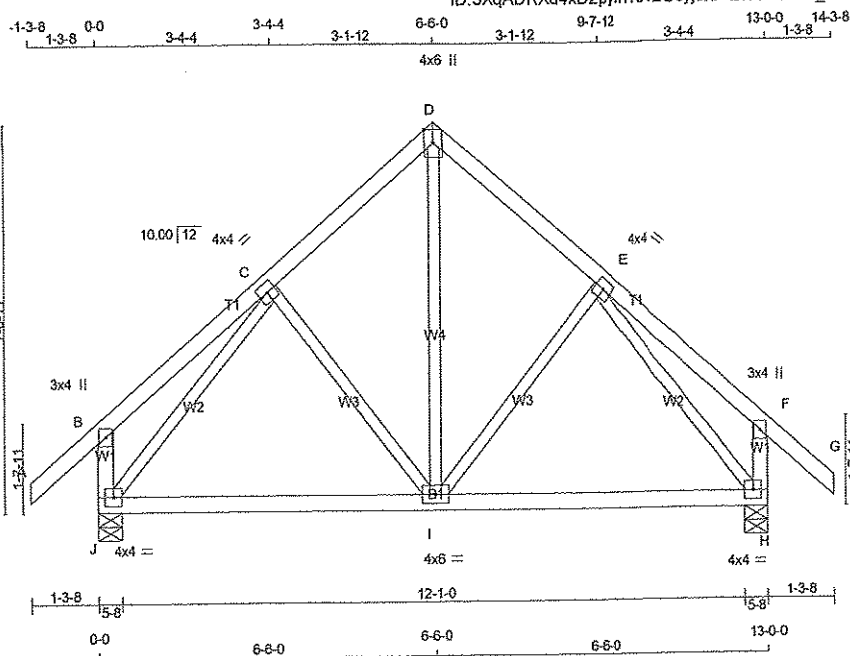
WFO NO. TAM 43529-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
282110	T14	3	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:3XqADRXd4xD2pyIhTJT2Ceyj0xP-E7AKE?HV_9NOx5bG7QnVX9Bt8phZIFz?H62KnylyFv



Scale = 1:42.5

TOTAL WEIGHT = 3 X 62 = 186 lb

LUMBER				DESCR	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	SPF	
A - D	2x4	DRY	No.2	SPF	
D - G	2x4	DRY	No.2	SPF	
J - B	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVWV-I	MT20	4.0	4.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMVWV-I	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVWV-I	MT20	4.0	4.0		
I	BMVWV-I	MT20	4.0	6.0		
J	BMVWV-I	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT		VERT	HORZ	DOWN	HORZ
J		846	0	846	0
H		846	0	846	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS	DEAD	SOIL
JT	COMBINED	SNOW	LIVE	PERM. LIVE
J	678	403 / 0	137 / 0	0 / 0
H	678	403 / 0	137 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)
FR-TO		FROM	TO	FR-TO	
A-B	0 / 37	-84.3	-84.3 0.12 (1)	I-D	0 / 407
B-C	0 / 20	-84.3	-84.3 0.14 (1)	I-E	-118 / 39
C-D	-506 / 0	-84.3	-84.3 0.11 (1)	C-I	-118 / 39
D-E	-506 / 0	-84.3	-84.3 0.11 (1)	J-C	-714 / 0
E-F	0 / 20	-84.3	-84.3 0.14 (1)	E-H	-714 / 0
F-G	0 / 37	-84.3	-84.3 0.12 (1)		
J-B	-221 / 0	0.0	0.0 0.02 (1)		
H-F	-221 / 0	0.0	0.0 0.02 (1)		
J-I	0 / 449	-28.0	-28.0 0.39 (2)		
I-H	0 / 449	-28.0	-28.0 0.39 (2)		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
	DL = 3.0 PSF
BOT CH.	LL = 10.5 PSF
	DL = 7.0 PSF
TOTAL LOAD	= 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/999 (0.09")

CSI: TC=0.14 (B-C:1), BC=0.39 (I-J:2), WB=0.30 (C-J:1), SSI=0.15 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1658

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.81 (C) (INPUT = 0.90)
JSI METAL= 0.26 (C) (INPUT = 1.00)



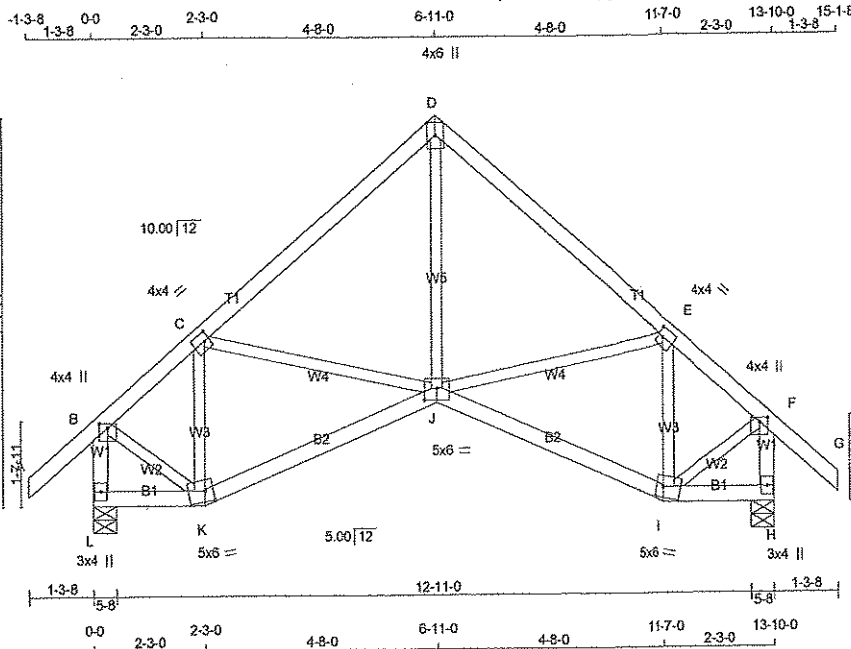
DRWG NO. TAM 4353011
STRUCTURAL
CONSULTANT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43854	DRWG NO.
282110	T15	3	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:3XqADRXd4xD2pyhTJT2Ceyj0xP-E7AKE?HV_9NOx5bGb7QnVX9AV8s0ZKsz?H62KnyiyFv



Scale = 1:44.4

TOTAL WEIGHT = 3 X 64 = 193 lb

LUMBER	CHORDS	SIZE	DRY	LUMBER	DESCR.
N. L. G. A. RULES					
A - D	2x4	DRY	No.2	SPF	
D - G	2x4	DRY	No.2	SPF	
L - B	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
L - K	2x4	DRY	No.2	SPF	
K - J	2x4	DRY	No.2	SPF	
J - I	2x4	DRY	No.2	SPF	
I - H	2x4	DRY	No.2	SPF	

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMVW-t	MT20	4.0	4.0	2.00	1.25
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMVW-t	MT20	4.0	4.0	2.00	1.25
F	TMVW+p	MT20	4.0	4.0	1.00	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BBWW-m	MT20	5.0	6.0	2.50	2.00
J	BBWW-w	MT20	5.0	6.0	2.75	3.00
K	BBWW-m	MT20	5.0	6.0	2.50	2.00
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS							
	FACTORED		MAXIMUM FACTORED			INPUT	REQD
	GROSS REACTION		GROSS REACTION			BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
L	893	0	893	0	0	5-8	5-8
H	893	0	893	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST CASE		MAX/MIN COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	717	425 / 0	145 / 0	0 / 0	0 / 0	147 / 0	0 / 0
H	717	425 / 0	145 / 0	0 / 0	0 / 0	147 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED HORIZ. LOAD (LC1)	MAX. FACTORED HORIZ. LOAD (LC2)	MAX. FACTORED HORIZ. LOAD (LC3)	MAX. FACTORED HORIZ. LOAD (LC4)	MAX. FACTORED HORIZ. LOAD (LC5)	MAX. FACTORED HORIZ. LOAD (LC6)	MAX. FACTORED HORIZ. LOAD (LC7)	MAX. FACTORED HORIZ. LOAD (LC8)	MAX. FACTORED HORIZ. LOAD (LC9)	MAX. FACTORED HORIZ. LOAD (LC10)
FR-TO													
A-B	0 / 37	-84.3	-84.3	0.12 (1)	10.00	J-D	0 / 545	0.12 (1)					
B-C	-627 / 0	-84.3	-84.3	0.18 (1)	6.25	J-E	0 / 32	0.01 (2)					
C-D	-707 / 0	-84.3	-84.3	0.23 (1)	6.25	I-E	-416 / 0	0.08 (1)					
D-E	-707 / 0	-84.3	-84.3	0.23 (1)	6.25	C-J	0 / 32	0.01 (2)					
E-F	-627 / 0	-84.3	-84.3	0.18 (1)	6.25	K-C	-416 / 0	0.08 (1)					
F-G	0 / 37	-84.3	-84.3	0.12 (1)	10.00	B-K	0 / 601	0.14 (1)					
L-B	-862 / 0	0.0	0.0	0.09 (1)	7.81	I-F	0 / 601	0.14 (1)					
H-F	-862 / 0	0.0	0.0	0.09 (1)	7.81								
L-K	0 / 0	-28.0	-28.0	0.04 (2)	10.00								
K-J	0 / 549	-28.0	-28.0	0.24 (2)	10.00								
J-I	0 / 549	-28.0	-28.0	0.24 (2)	10.00								
I-H	0 / 0	-28.0	-28.0	0.04 (2)	10.00								

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.46")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.46")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.07")

CSI: TC=0.23 (D-E:1), BC=0.24 (I-J:2), WB=0.14 (F-I:1), SSI=0.14 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

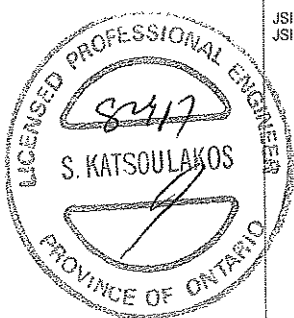
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1687
	822	2284	1656

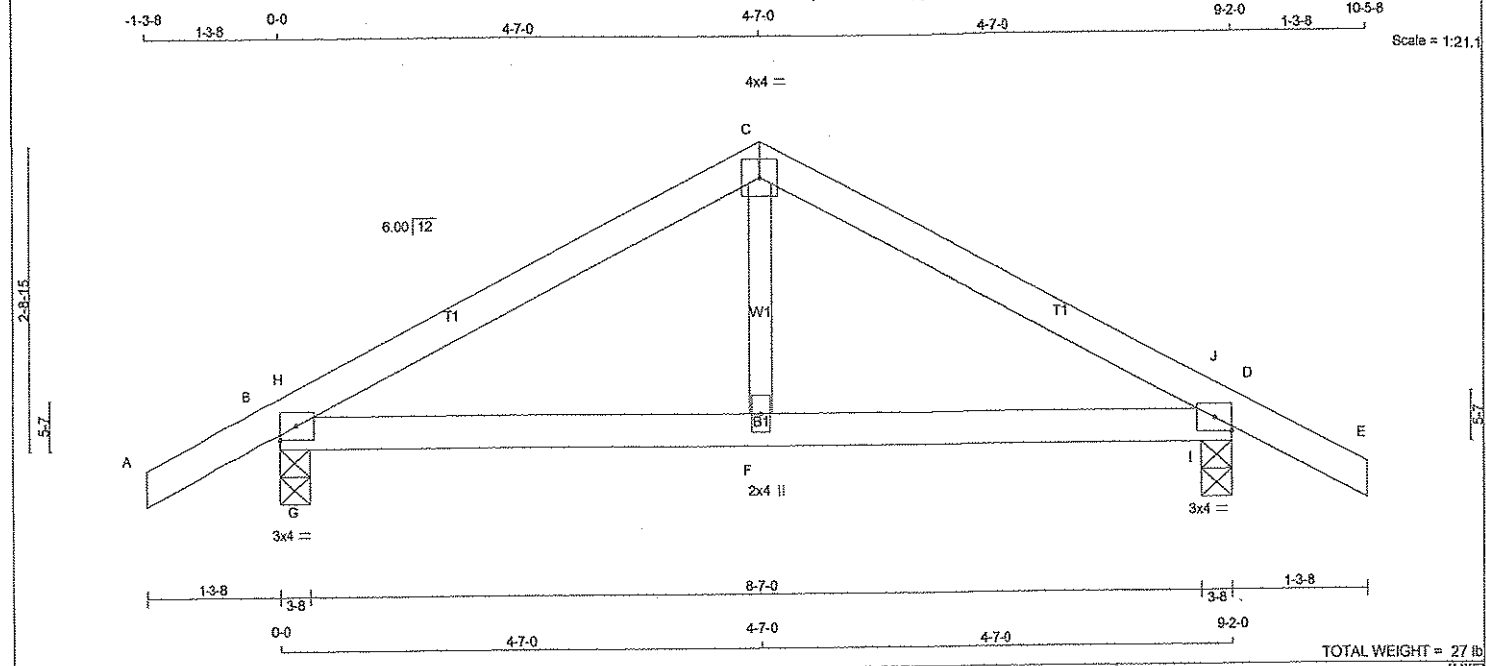
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (I) (INPUT = 0.90)
JSI METAL= 0.20 (B) (INPUT = 1.00)



DRWG NO. TAM 43531-17
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		Edge
C	TTW-p	MT20	4.0	4.0		
D	TMB1-I	MT20	3.0	4.0		Edge
F	BMW-w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
B	612	0	612	0	3-8	3-8
I(D)	646	0	646	0	3-8	3-8

UNFACTORED REACTIONS							
JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	485	296 / 0	94 / 0	0 / 0	0 / 0	97 / 0	0 / 0
I(D)	514	312 / 0	99 / 0	0 / 0	0 / 0	103 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, I(D)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO	LENGTH	FR-TO		LENGTH	
A-B	0 / 21	-84.3 -84.3	0.11 (1)	10.00	F-C	0 / 247	0.06 (2)
B-H	-505 / 0	-84.3 -84.3	0.10 (1)	6.25	G-H	-301 / 213	0.00 (1)
H-C	-552 / 0	-84.3 -84.3	0.21 (1)	6.25	I-J	-598 / 86	0.00 (1)
C-J	-559 / 0	-84.3 -84.3	0.18 (1)	6.25			
J-D	-385 / 0	-84.3 -84.3	0.17 (1)	6.25			
D-E	0 / 21	-84.3 -84.3	0.11 (1)	10.00			
B-G	0 / 485	-28.0 -28.0	0.19 (1)	10.00			
G-F	0 / 485	-28.0 -28.0	0.20 (1)	10.00			
F-I	0 / 485	-28.0 -28.0	0.19 (2)	10.00			
I-D	0 / 485	-28.0 -28.0	0.13 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.31")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.02")
ALLOWABLE DEFL.(TL) = $L/360$ (0.31")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.04")

CSI: TC=0.21 (C-H:1), BC=0.20 (F-G:1), WB=0.06 (C-F:2), SSI=0.35 (D-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.56 (B) (INPUT = 0.90)
JSI METAL= 0.20 (D) (INPUT = 1.00)



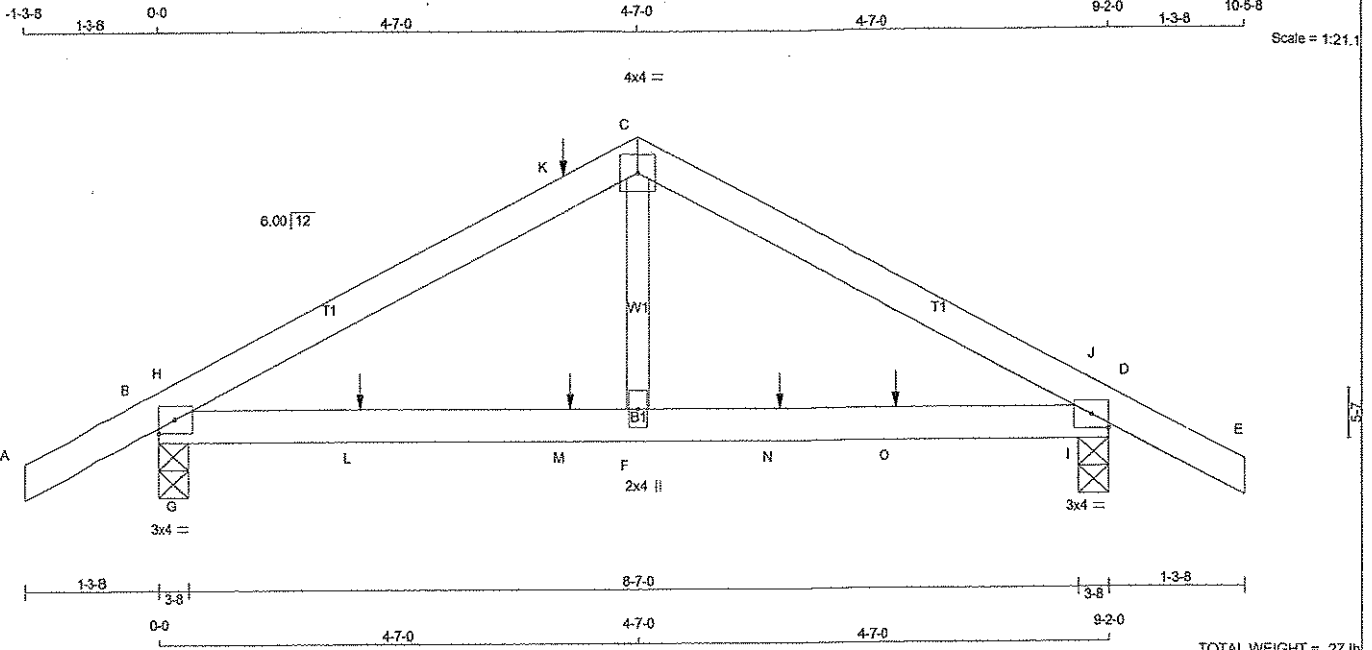
DRWG NO. TAM 4353217
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43854	DRWG NO.
282110	T17Z	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 Mitek Industries, Inc. Tue Aug 29 16:11:48 2017 Page 1

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LUMBER				DESCR.			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	SPF	SPF	SPF	SPF
A - C	2x4	DRY	No.2	SPF			
C - E	2x4	DRY	No.2	SPF			
B - D	2x4	DRY	No.2	SPF			
ALL WEBS	2x3	DRY	No.2	SPF			
DRY: SEASONED LUMBER.							

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y
B	TMB1-I	MT20	3.0	4.0	Edge
C	TTW-p	MT20	4.0	4.0	
D	TMB1-I	MT20	3.0	4.0	Edge
F	BMW+w	MT20	2.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 0.1 lbs FACTORED DOWN AT 3-11-4 ON TOP CHORD, AND AT 1-11-4, AT 3-11-4, AND AT 5-11-4, AND AT 7-1-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	IN-SX	IN-SX
B	612	0	612	0	0	3-8	3-8		
I(D)	646	0	646	0	0	3-8	3-8		

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN. COMPONENT REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
B	486	286 / 0	94 / 0	0 / 0	0 / 0	97 / 0	0 / 0		
I(D)	514	312 / 0	99 / 0	0 / 0	0 / 0	103 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, I(D)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD LC1 (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD LC1 (LBS)	MAX. FACTORED VERT. LOAD LC1 (LBS)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 21	-84.3	-84.3	0.12 (1)	10.00	F-C	0 / 247
B-H	-505 / 0	-84.3	-84.3	0.14 (1)	6.25	G-H	-301 / 213
H-K	-552 / 0	-84.3	-84.3	0.24 (1)	6.25	I-J	-598 / 65
K-C	-552 / 0	-84.3	-84.3	0.24 (1)	6.25		
C-J	-559 / 0	-84.3	-84.3	0.25 (1)	6.25		
J-D	-368 / 0	-84.3	-84.3	0.24 (1)	6.25		
D-E	0 / 21	-84.3	-84.3	0.12 (1)	10.00		
B-G	0 / 485	-28.0	-28.0	0.21 (1)	10.00		
G-L	0 / 485	-28.0	-28.0	0.22 (1)	10.00		
L-M	0 / 485	-28.0	-28.0	0.22 (1)	10.00		
M-F	0 / 485	-28.0	-28.0	0.22 (1)	10.00		
F-N	0 / 485	-28.0	-28.0	0.21 (2)	10.00		
N-O	0 / 485	-28.0	-28.0	0.21 (2)	10.00		
O-I	0 / 485	-28.0	-28.0	0.21 (2)	10.00		
I-D	0 / 485	-28.0	-28.0	0.14 (2)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE
K	3-11-4	---	---	---	BACK	VERT	TOTAL
L	1-11-4	---	---	---	BACK	VERT	TOTAL
M	3-11-4	---	---	---	BACK	VERT	TOTAL
N	5-11-4	---	---	---	BACK	VERT	TOTAL
O	7-1-4	---	---	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.31")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.02")
ALLOWABLE DEFL.(TL) = $L/360$ (0.31")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.04")

CSI: TC=0.25 (C-J:1), BC=0.22 (F-G:1), WB=0.08 (C-F:2), SSI=0.39 (D-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

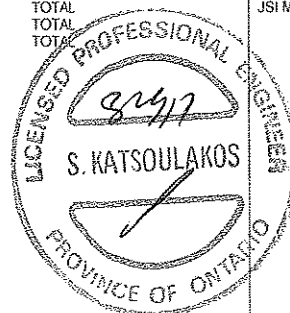
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	618	354	1687
	MAX	MIN	MAX
	618	354	1687

PLATE PLACEMENT TOL. = 0.250 inches

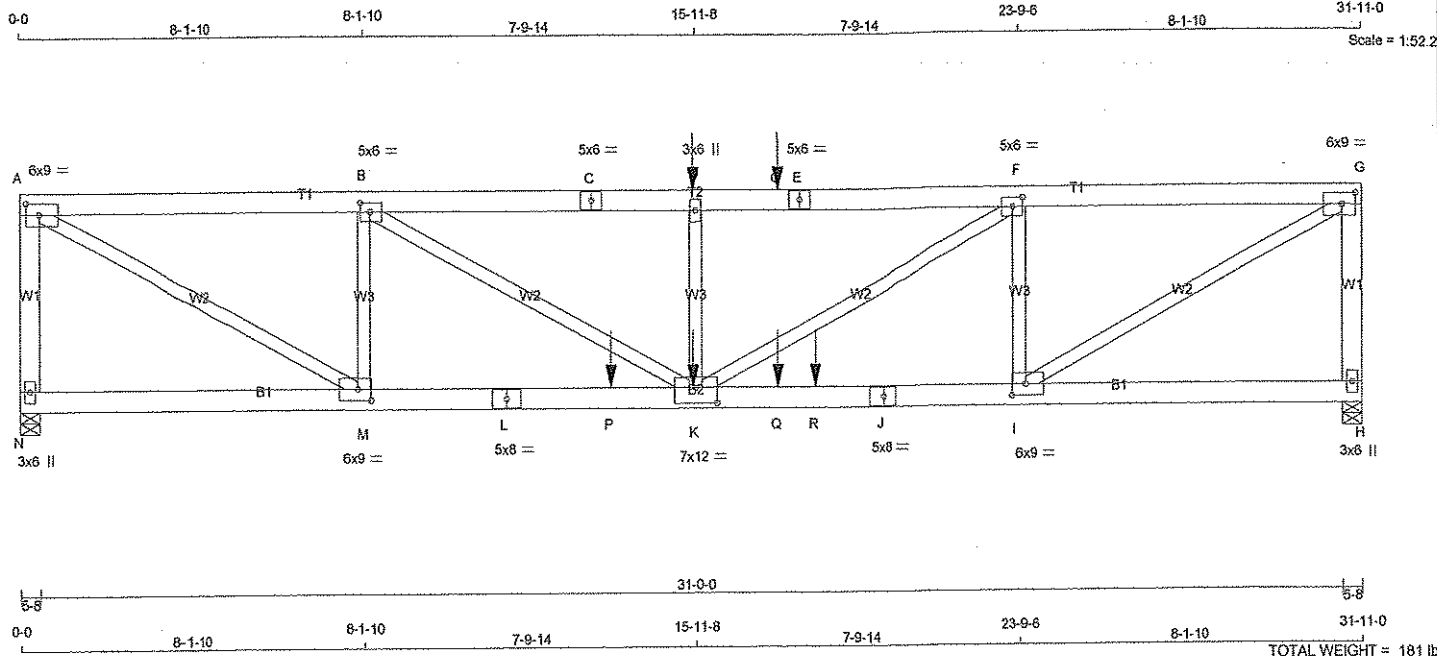
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.56 (B) (INPUT = 0.90)
JSI METAL= 0.20 (D) (INPUT = 1.00)



DWG NO. T17Z 4353317
STRUCTURAL
COMPONENT ONLY

Tamarack Roof Truss, Burlington



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
N - A	2x6	DRY	No.2
A - C	2x6	DRY	1650F 1.5E
C - E	2x6	DRY	1650F 1.5E
E - G	2x6	DRY	1650F 1.5E
H - G	2x6	DRY	No.2
N - L	2x6	DRY	1650F 1.5E
L - J	2x6	DRY	1650F 1.5E
J - H	2x6	DRY	1650F 1.5E

ALL WEBS 2x4 DRY No.2 SPF
 DRY: SEASONED LUMBER

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWV-I	MT20	6.0	9.0	3.00	3.75
B	TMWV-I	MT20	5.0	6.0	2.50	2.75
C	TS-I	MT20	5.0	6.0		
D	TMWV-I	MT20	3.0	6.0		
E	TS-I	MT20	5.0	6.0		
F	TMWV-I	MT20	5.0	6.0	2.50	2.75
G	TMWV-I	MT20	6.0	9.0	3.00	3.75
H	BMV1+p	MT20	3.0	6.0		
I	BMWV-I	MT20	6.0	9.0	3.00	3.75
J	BS-I	MT20	5.0	6.0		
K	BMWV-I	MT20	7.0	12.0	4.25	6.00
L	BS-I	MT20	5.0	6.0		
M	BMWV-I	MT20	6.0	9.0	3.00	3.75
N	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 38.3 lbs FACTORED DOWN AT 15-10-12, AND 38.3 lbs FACTORED DOWN AT 17-10-12 ON TOP CHORD, AND 898.3 lbs FACTORED DOWN AT 13-11-8, 34.1 lbs FACTORED DOWN AT 15-10-12, AND 34.1 lbs FACTORED DOWN AT 17-10-12, AND 650.2 lbs FACTORED DOWN AT 18-9-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED		MAXIMUM FACTORED		INPUT	REQD
	GROSS REACTION		GROSS REACTION		BRG	
JT	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX
N	2617	0	2617	0	5-8	5-8
H	2630	0	2630	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN. COMPONENT REACTIONS		DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE		
N	2120	1223 / 0	452 / 0	0 / 0	445 / 0	0 / 0
H	2130	1229 / 0	454 / 0	0 / 0	447 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.74 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		WEBS		MAX. FACTORED	
	MAX. FORCE	VERT. LOAD	LC1	MAX	MAX. MEMB.	FORCE	MAX	CSI (LC)
FR-TO	(LBS)	FROM TO	(PLF)	CSI (LC)	UNBRAC	(LBS)	CSI (LC)	
N-A	-2540 / 0	0.0	0.0	0.55 (1)	6.46	I-G	0 / 4775	0.84 (1)
A-B	-4170 / 0	-84.3	-84.3	0.38 (1)	4.46	A-M	0 / 4744	0.84 (1)
B-C	-6111 / 0	-84.3	-84.3	0.45 (1)	3.75	I-F	-1860 / 0	0.42 (1)
C-D	-6111 / 0	-84.3	-84.3	0.45 (1)	3.75	M-B	-1862 / 0	0.42 (1)
D-O	-6111 / 0	-84.3	-84.3	0.45 (1)	3.74	K-F	0 / 2197	0.39 (1)
O-E	-6111 / 0	-84.3	-84.3	0.45 (1)	3.74	B-K	0 / 2228	0.39 (1)
E-F	-6111 / 0	-84.3	-84.3	0.45 (1)	3.74	K-D	-661 / 0	0.15 (1)
F-G	-4197 / 0	-84.3	-84.3	0.37 (1)	4.44			
H-G	-2553 / 0	0.0	0.0	0.55 (1)	6.45			

N-M	0 / 0	-28.0	-28.0	0.20 (2)	10.00
M-L	0 / 4170	-28.0	-28.0	0.85 (1)	10.00
L-P	0 / 4170	-28.0	-28.0	0.85 (1)	10.00
P-K	0 / 4170	-28.0	-28.0	0.85 (1)	10.00
K-Q	0 / 4197	-28.0	-28.0	0.85 (1)	10.00
Q-R	0 / 4197	-28.0	-28.0	0.85 (1)	10.00
R-J	0 / 4197	-28.0	-28.0	0.85 (1)	10.00
J-I	0 / 4197	-28.0	-28.0	0.85 (1)	10.00
I-H	0 / 0	-28.0	-28.0	0.21 (2)	10.00

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
D	15-10-12	-38	-38		BACK	VERT	TOTAL
K	15-10-12	-20	-34		BACK	VERT	TOTAL
O	17-10-12	-38	-38		BACK	VERT	TOTAL
P	13-11-8	-888	-888		BACK	VERT	TOTAL
Q	17-10-12	-20	-34		BACK	VERT	TOTAL
R	18-9-8	-660	-660		BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 3.0 PSF
BOT CH. LL	= 10.5 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (1.06")
 CALCULATED VERT. DEFL. (LL) = $L/999$ (0.34")
 ALLOWABLE DEFL. (TL) = $L/360$ (1.06")
 CALCULATED VERT. DEFL. (TL) = $L/708$ (0.54")

CSI: TC=0.55 (G-H-1), BC=0.85 (H-K-1), WB=0.84 (G-I-1), SSI=0.57 (K-M-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

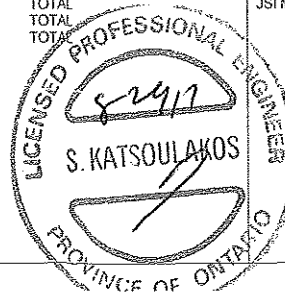
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	618	354	1667
	822	2284	1658

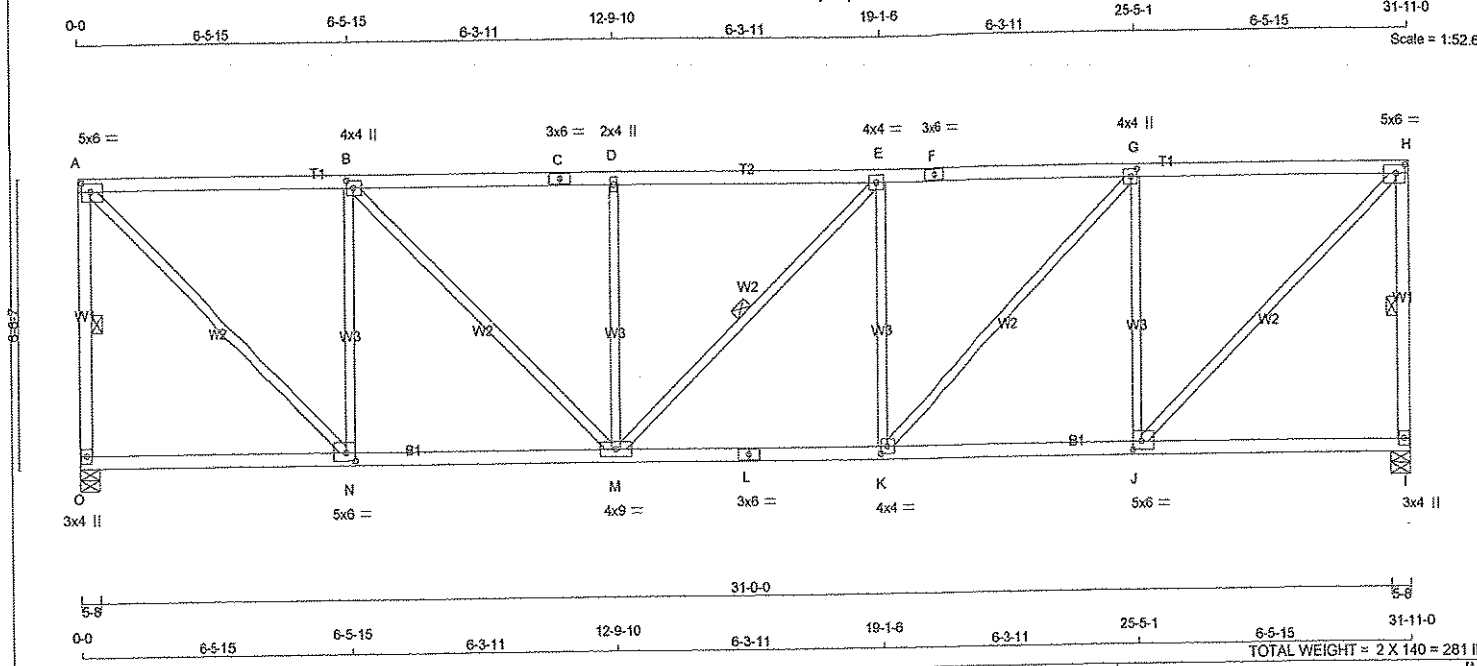
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (G) (INPUT = 0.90)
 JSI METAL = 0.98 (J) (INPUT = 1.00)



DRWG NO. TAM 4353617
 STRUCTURAL
 COMPONENT ONLY



LUMBER				BUILDING DESIGNER			
N. L. G. A. RULES				BEARINGS			
CHORDS	SIZE	LUMBER	DESCR.	FACTORED	MAXIMUM FACTORED	INPUT	REQD
O - A	2x4	DRY	No.2	GROSS REACTION	GROSS REACTION	BRG	BRG
A - C	2x4	DRY	No.2	VERT	HORZ	DOWN	IN-SX
C - F	2x4	DRY	No.2	1791	0	1791	0
F - H	2x4	DRY	No.2	1791	0	1791	0
I - H	2x4	DRY	No.2				
O - L	2x4	DRY	No.2				
L - I	2x4	DRY	No.2				
ALL WEBS	2x3	DRY	No.2				
DRY: SEASONED LUMBER.							

PLATES (table in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-I	MT20	5.0	6.0	2.25	2.50
B	TMVW+I	MT20	4.0	4.0	2.00	1.75
C	TS-I	MT20	3.0	6.0		
D	TMVW+I	MT20	2.0	4.0		
E	TMVW-I	MT20	4.0	4.0		
F	TS-I	MT20	3.0	6.0		
G	TMVW+I	MT20	4.0	4.0	2.00	1.75
H	TMVW-I	MT20	5.0	6.0	2.25	2.50
I	BMV1+p	MT20	3.0	4.0		
J	BMVW-I	MT20	5.0	6.0	2.25	2.50
K	BMVW-I	MT20	4.0	4.0	2.00	1.75
L	BS-I	MT20	3.0	6.0		
M	BMVW+I	MT20	4.0	9.0		
N	BMVW-I	MT20	5.0	6.0	2.25	2.50
O	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER							
UNFACTORED REACTIONS							
JT	1ST LCASE	MAX MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL	
O	1471	817 / 0	335 / 0	0 / 0	0 / 0	319 / 0	0 / 0
I	1471	817 / 0	335 / 0	0 / 0	0 / 0	319 / 0	0 / 0

UNFACTORED REACTIONS							
JT	1ST LCASE	MAX MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL	
O	1471	817 / 0	335 / 0	0 / 0	0 / 0	319 / 0	0 / 0
I	1471	817 / 0	335 / 0	0 / 0	0 / 0	319 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.81 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF A-O, H-I, E-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

CHORDS				WEBS			
MEMB.	MAX FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)	MAX UNBRAC LENGTH (LC)	MEMB.	MAX FACTORED FORCE (LBS)	MAX LC1 (LC)
FR-TO		FROM	TO		FR-TO		
O-A	-1718 / 0	0.0	0.0	0.34 (1)	5.08	J-H	0 / 2163
A-B	-1560 / 0	-84.3	-84.3	0.61 (1)	4.52	A-N	0 / 2164
B-C	-2252 / 0	-84.3	-84.3	0.69 (1)	3.81	J-G	-1297 / 0
C-D	-2252 / 0	-84.3	-84.3	0.69 (1)	3.81	N-B	-1296 / 0
D-E	-2252 / 0	-84.3	-84.3	0.53 (1)	4.07	K-G	0 / 976
E-F	-2253 / 0	-84.3	-84.3	0.69 (1)	3.81	B-M	0 / 974
F-G	-2253 / 0	-84.3	-84.3	0.69 (1)	3.81	K-E	-515 / 0
G-H	-1559 / 0	-84.3	-84.3	0.61 (1)	4.52	M-D	-514 / 0
I-H	-1718 / 0	0.0	0.0	0.34 (1)	5.08	M-E	-2 / 0
O-N	0 / 0	-28.0	-28.0	0.28 (3)	10.00		
N-M	0 / 1560	-28.0	-28.0	0.47 (2)	10.00		
M-L	0 / 2253	-28.0	-28.0	0.47 (1)	10.00		
L-K	0 / 2253	-28.0	-28.0	0.47 (1)	10.00		
K-J	0 / 1559	-28.0	-28.0	0.47 (2)	10.00		
J-I	0 / 0	-28.0	-28.0	0.28 (3)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.06")
CALCULATED VERT. DEFL.(LL)= L/999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (1.06")
CALCULATED VERT. DEFL.(TL)= L/999 (0.27")

CSI: TC=0.69 (B-D:1), BC=0.47 (K-M:1), WB=0.91 (G-J:1), SSI=0.26 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.88 (A) (INPUT = 0.90)
JSI METAL= 0.70 (L) (INPUT = 1.00)

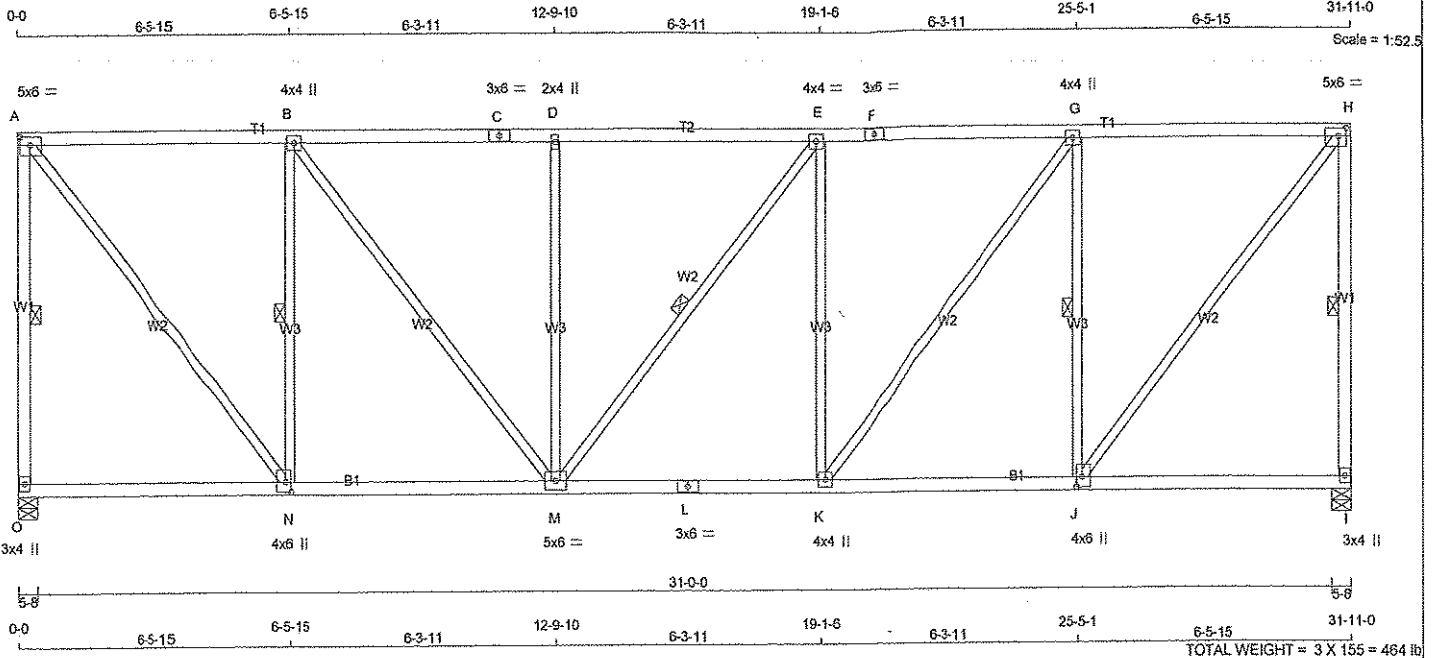
DWG NO. YAM 4353717
STRUCTURAL
COMPONENT ONLY



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
282111	T22	3	1	TRUSS DESC.		

Tamareck Roof Truss, Burlington

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TOTAL WEIGHT = 3 X 155 = 464 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
O - A	2x4	DRY	No.2
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - I	2x4	DRY	No.2
O - L	2x4	DRY	No.2
L - I	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	HORZ	BRG	IN-SX	IN-SX	
O	1791	0	1791	0	0	5-8	5-8	5-8	
I	1791	0	1791	0	0	5-8	5-8	5-8	

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	
O	1471	817/0	335/0	0/0	0/0	319/0	0/0	
I	1471	817/0	335/0	0/0	0/0	319/0	0/0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.26 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF A-O, H-I, G-J, B-N, E-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MEMB.	FORCE (LBS)	MAX. FACTORED (LBS)	MAX. CSI (LC)
FR-TO		FROM	TO	FR-TO			
O-A	-1718/0	0.0	0.0 0.52 (1)	5.08	J-H	0/1940	0.44 (1)
A-B	-1232/0	-84.3	-84.3 0.58 (1)	4.98	A-N	0/1941	0.44 (1)
B-C	-1778/0	-84.3	-84.3 0.63 (1)	4.26	J-G	-1297/0	0.55 (1)
C-D	-1778/0	-84.3	-84.3 0.63 (1)	4.26	N-B	-1296/0	0.55 (1)
D-E	-1778/0	-84.3	-84.3 0.47 (1)	4.52	K-G	0/878	0.20 (1)
E-F	-1779/0	-84.3	-84.3 0.63 (1)	4.26	B-M	0/876	0.20 (1)
F-G	-1779/0	-84.3	-84.3 0.63 (1)	4.26	K-E	-515/0	0.63 (1)
G-H	-1231/0	-84.3	-84.3 0.57 (1)	4.99	M-D	-514/0	0.67 (1)
H-I	-1718/0	0.0	0.0 0.52 (1)	5.08	M-E	-2/0	0.03 (1)
O-N	0/0	-28.0	-28.0 0.29 (3)	10.00			
N-M	0/1232	-28.0	-28.0 0.43 (2)	10.00			
M-L	0/1779	-28.0	-28.0 0.41 (2)	10.00			
L-K	0/1779	-28.0	-28.0 0.41 (2)	10.00			
K-J	0/1231	-28.0	-28.0 0.43 (2)	10.00			
J-I	0/0	-28.0	-28.0 0.29 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 3.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (1.06")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.14")
ALLOWABLE DEFL. (TL) = $L/360$ (1.06")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.23")

CSI: TC=0.63 (B-D:1), BC=0.43 (M-N:2), WB=0.68 (E-K:1), SSI=0.28 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX MIN
MT20	618	354	1687 822 2284 1655

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.90 (A) (INPUT = 0.90)
JSI METAL= 0.58 (L) (INPUT = 1.00)



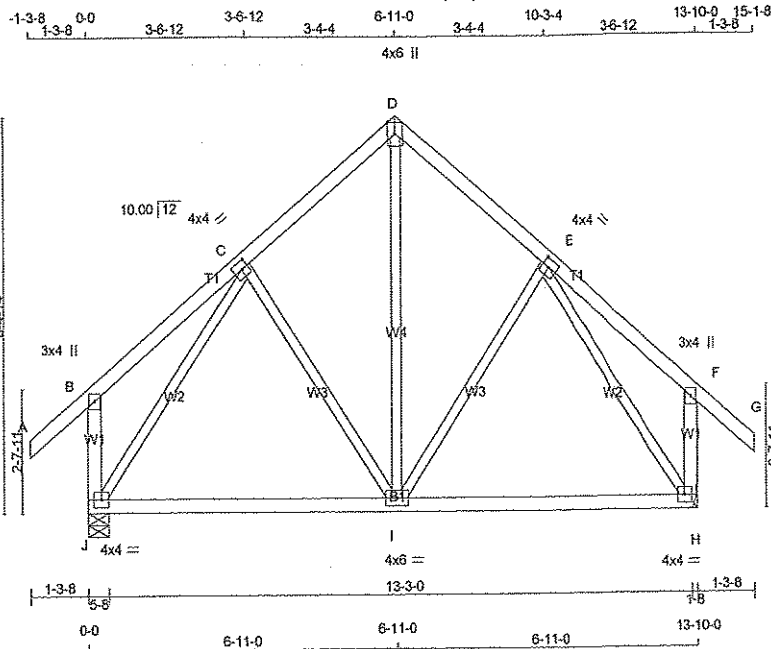
DRWG NO. TAM 43538-1
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
282111	T23	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1/49.6

TOTAL WEIGHT = 71 lb

[M/JF]

LUMBER	CHORDS	SIZE	DRY	LUMBER	DESCR.
N. L. G. A. RULES					
A - D	2x4	DRY	No.2	SPF	
D - G	2x4	DRY	No.2	SPF	
J - B	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	4.0	2.00	1.75
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMVW1-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
J	893	0	893	0
H	893	0	893	0

HANGER BY OTHERS
MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX / MIN	COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
J	717	425 / 0	145 / 0	0 / 0	0 / 0	147 / 0	0 / 0	0 / 0	0 / 0
H	717	425 / 0	145 / 0	0 / 0	0 / 0	147 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)
FR-TO				FR-TO			
A-B	0 / 37	-84.3	-84.3 0.12 (1)	10.00	I-D	0 / 374	0.08 (2)
B-C	0 / 22	-84.3	-84.3 0.16 (1)	10.00	I-E	-72 / 64	0.05 (1)
C-D	-480 / 0	-84.3	-84.3 0.12 (1)	6.25	C-I	-72 / 64	0.05 (1)
D-E	-480 / 0	-84.3	-84.3 0.12 (1)	6.25	J-C	-711 / 0	0.47 (1)
E-F	0 / 22	-84.3	-84.3 0.16 (1)	10.00	E-H	-711 / 0	0.47 (1)
F-G	0 / 37	-84.3	-84.3 0.12 (1)	10.00			
J-B	-227 / 0	0.0	0.0 0.03 (1)	7.81			
H-F	-227 / 0	0.0	0.0 0.03 (1)	7.81			
J-I	0 / 393	-28.0	-28.0 0.43 (2)	10.00			
I-H	0 / 393	-28.0	-28.0 0.43 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 3.0 PSF
BOT CH. LL	= 10.5 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.46")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.07")
ALLOWABLE DEFL.(TL) = $L/360$ (0.46")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.11")

CSI: TC=0.18 (B-C-1), BC=0.43 (I-J-2), WB=0.47 (E-H-1), SSI=0.16 (I-J-3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

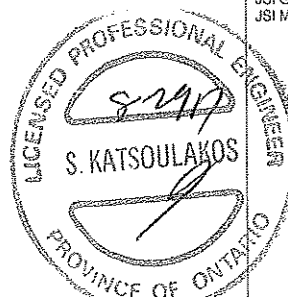
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

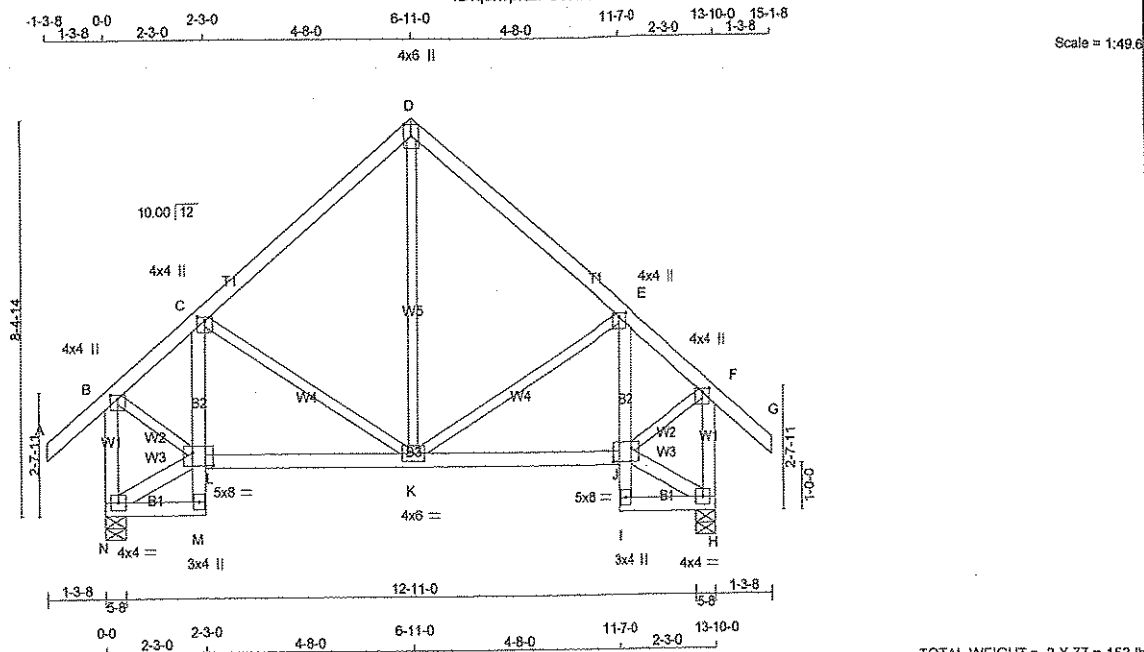
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.84 (C) (INPUT = 0.90)
JSI METAL= 0.26 (E) (INPUT = 1.00)



BWG NO. TAM 4353917
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = $2 \times 77 = 153 \text{ lb}$

N. L. G. A. RULES

CHORDS	SIZE	LUMBER
A - D	2x4	DRY No.2
D - G	2x4	DRY No.2
N - B	2x4	DRY No.2
H - F	2x4	DRY No.2
N - M	2x4	DRY No.2
M - C	2x4	DRY No.2
L - J	2x4	DRY No.2
I - E	2x4	DRY No.2
I - H	2x4	DRY No.2

ALL WEBS EXCEPT	2x3	DRY	No.2
N - L	2x4	DRY	No.2
J - H	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	CAE.F					
B	TTWV+p	MT20	4.0	4.0	1.00	2.00
D	TTV+p	MT20	4.0	6.0		Edge
H	BMVW1-t	MT20	4.0	4.0		
I	BMV+p	MT20	3.0	4.0		
J	BMVWV-t	MT20	5.0	8.0	3.25	5.50
K	BMVWVW-t	MT20	4.0	6.0		
L	BMVWV-t	MT20	5.0	8.0	3.25	5.50
M	BMV+p	MT20	3.0	4.0		
N	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
N	893	0	893	0	0	5-8	5-8
H	893	0	893	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	717	425/0	145/0	0/0	0/0	147/0	0/0
H	717	425/0	145/0	0/0	0/0	147/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MAX. FACTORED		FACTORED			MAX. FACTORED		MAX. FACTORED		
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX. CSI (LC)		
FR-TO		FROM TO			FR-TO				
A-B	0/37	-84.3	-84.3	0.12 (1)	10.00	K-D	0/354	0.08 (2)	
B-C	-627/0	-84.3	-84.3	0.19 (1)	6.25	K-E	-161/0	0.09 (1)	
C-D	-532/0	-84.3	-84.3	0.23 (1)	6.25	C-K	-161/0	0.09 (1)	
D-E	-532/0	-84.3	-84.3	0.23 (1)	6.25	N-L	-12/0	0.00 (1)	
E-F	-627/0	-84.3	-84.3	0.19 (1)	6.25	B-L	0/603	0.14 (1)	
F-G	0/37	-84.3	-84.3	0.12 (1)	10.00	J-H	-12/0	0.00 (1)	
N-B	-858/0	0.0	0.0	0.12 (1)	7.81	J-F	0/603	0.14 (1)	
H-F	-858/0	0.0	0.0	0.12 (1)	7.81				
N-M	0/11	-28.0	-28.0	0.04 (2)	10.00				
M-L	0/52	0.0	0.0	0.02 (1)	10.00				
L-C	-215/6	0.0	0.0	0.01 (1)	7.81				
L-K	0/526	-28.0	-28.0	0.25 (2)	10.00				
K-J	0/526	-28.0	-28.0	0.25 (2)	10.00				
I-J	0/52	0.0	0.0	0.02 (1)	10.00				
J-E	-215/6	0.0	0.0	0.01 (1)	7.81				
I-H	0/11	-28.0	-28.0	0.04 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD =		46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= 1/360 (0.46")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.02")
ALLOWABLE DEFL.(TL)= 1/360 (0.46")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.04")

CSI: TC=0.23 (C-D:1), BC=0.25 (J-K:2), WB=0.14 (B-L:1), SSI=0.14 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1658

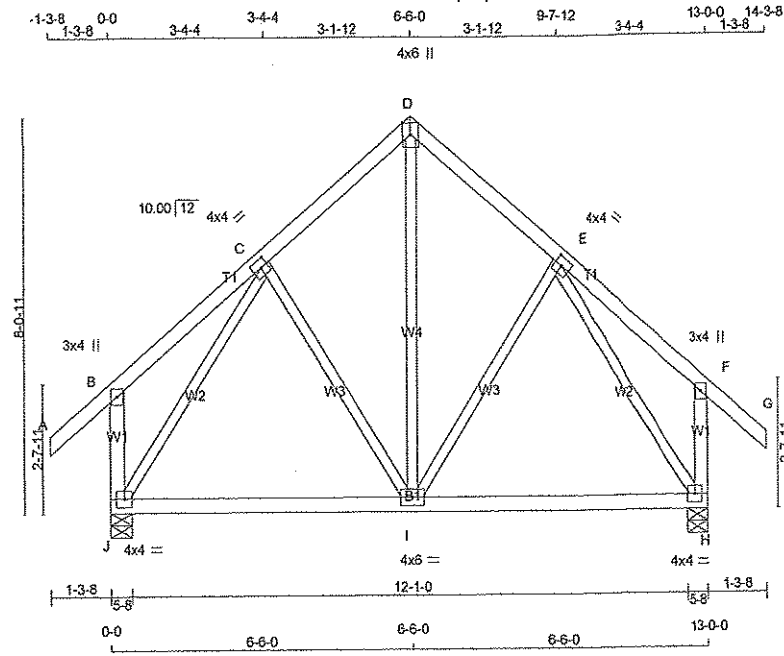
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg

JSI GRIP= 0.72 (B) (INPUT = 0.90)
JSI METAL= 0.20 (B) (INPUT = 1.00)



43540
STRUCTURAL
CONTENT ONLY



TOTAL WEIGHT = 5 X 68 = 339 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
J - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	4.0	2.00	1.75
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMVW1-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
J	846	0	846	0	5-8	5-8
H	846	0	846	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
J	678	403 / 0	137 / 0	0 / 0	0 / 0	138 / 0	0 / 0
H	678	403 / 0	137 / 0	0 / 0	0 / 0	138 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD LC1 (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD LC1 (LC)	
FR-TO		FROM TO		FR-TO				
A-B	0 / 37	-84.3	-84.3	0.12 (1)	10.00	I-D	0 / 342	0.03 (2)
B-C	0 / 20	-84.3	-84.3	0.14 (1)	10.00	I-E	-61 / 64	0.04 (1)
C-D	-443 / 0	-84.3	-84.3	0.11 (1)	6.25	C-I	-61 / 64	0.04 (1)
D-E	-443 / 0	-84.3	-84.3	0.11 (1)	6.25	J-C	-662 / 0	0.39 (1)
E-F	0 / 20	-84.3	-84.3	0.14 (1)	10.00	E-H	-662 / 0	0.39 (1)
F-G	0 / 37	-84.3	-84.3	0.12 (1)	10.00			
J-B	-221 / 0	0.0	0.0	0.03 (1)	7.81			
H-F	-221 / 0	0.0	0.0	0.03 (1)	7.81			
J-I	0 / 359	-28.0	-28.0	0.38 (2)	10.00			
I-H	0 / 359	-28.0	-28.0	0.38 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/999 (0.09")

CSI: TC=0.14 (B-C:1), BC=0.38 (H-I:2), WB=0.39 (C-J:1), SSI=0.15 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

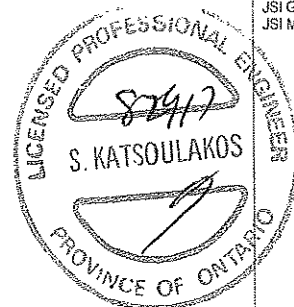
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	822	2284	1656

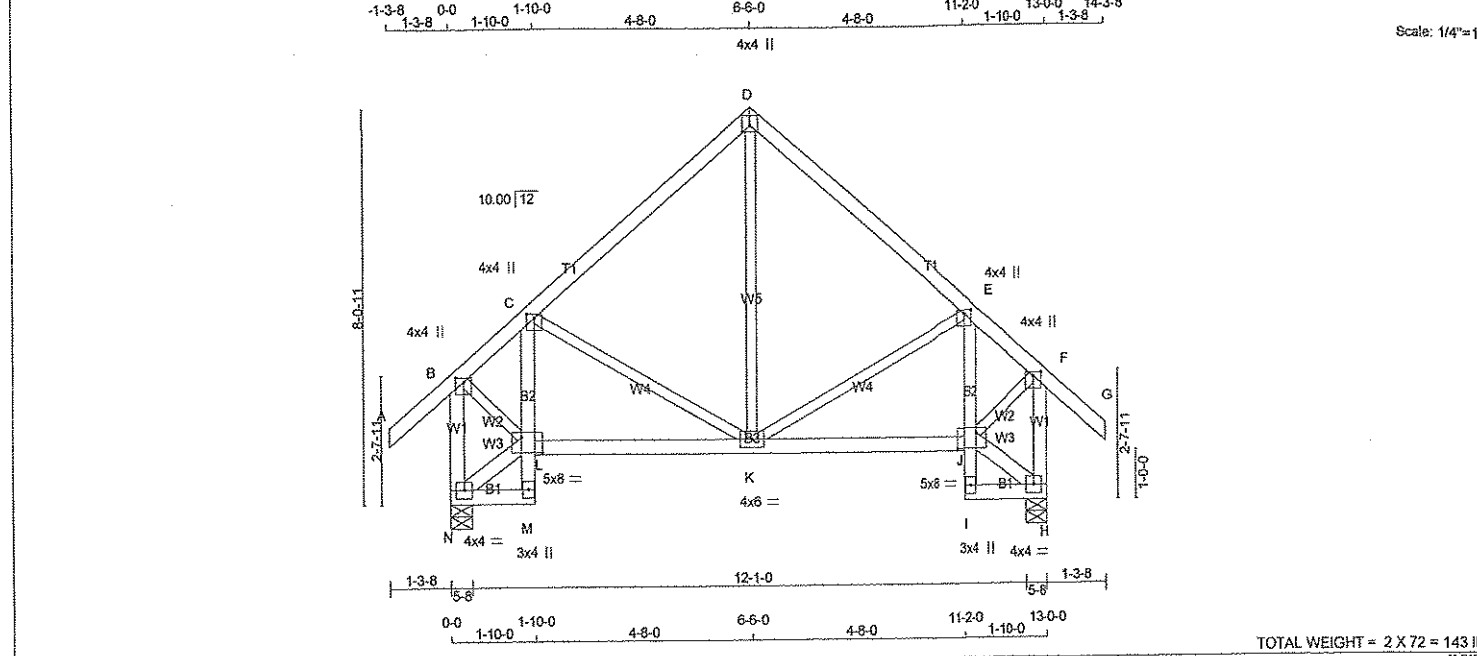
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.80 (E) (INPUT = 0.90)
JSI METAL= 0.24 (E) (INPUT = 1.00)



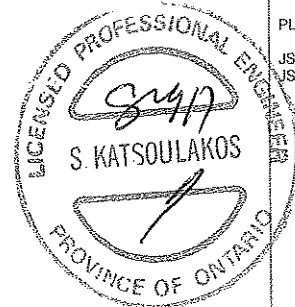
DRWNO. TAN 43541-17
STRUCTURAL
COMPONENT ONLY



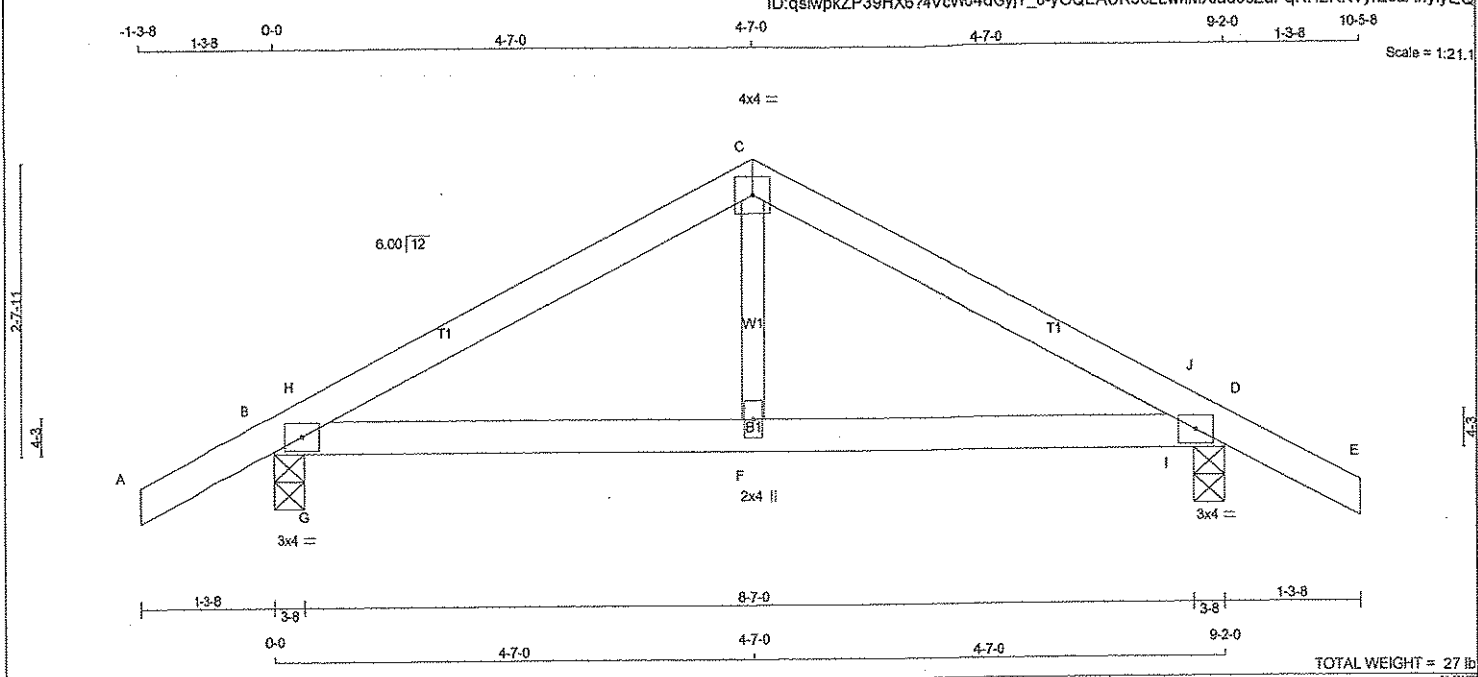
LUMBER				DESIGN CRITERIA			
N. L. G. A. RULES				SPECIFIED LOADS:			
CHORDS	SIZE	LUMBER	DESCR.	TOP CH.	LL	=	25.6 PSF
A - D	2x4	DRY	No.2	DL	=	3.0 PSF	
D - G	2x4	DRY	No.2	BOT CH.	LL	=	10.5 PSF
N - B	2x4	DRY	No.2	DL	=	7.0 PSF	
H - F	2x4	DRY	No.2	TOTAL LOAD	=	46.1 PSF	
N - M	2x4	DRY	No.2				
M - C	2x4	DRY	No.2				
L - J	2x4	DRY	No.2				
I - E	2x4	DRY	No.2				
I - H	2x4	DRY	No.2				
ALL WEBS 2x3 DRY No.2 SPF				SPACING = 24.0 IN. C/C			
EXCEPT				THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010			
N - L	2x4	DRY	No.2	THIS DESIGN COMPLIES WITH:			
J - H	2x4	DRY	No.2	- PART 9 OF OBC 2012, BCBC 2012, ABC 2014			
DRY: SEASONED LUMBER.				- CSA 086-09			
				- TPIC 2011			
				DESIGN ASSUMPTIONS			
				- OVERHANG NOT TO BE ALTERED OR CUT OFF.			
				(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD			
				ALLOWABLE DEFL. (LL) = L/360 (0.43")			
				CALCULATED VERT. DEFL. (LL) = L/999 (0.02")			
				ALLOWABLE DEFL. (TL) = L/360 (0.43")			
				CALCULATED VERT. DEFL. (TL) = L/999 (0.04")			
				CSI: TC=0.23 (C-D:1), BC=0.24 (K-L:2), WB=0.12 (B-L:1), SSF=0.14 (D-E:1)			
				DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10			
				COMP=1.10 SHEAR=1.10 TENS=1.10			
				COMPANION LIVE LOAD FACTOR = 0.50			
				TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.			
				NAIL VALUES			
				PLATE GRIP(DRY) SHEAR SECTION			
				(PSI) (PLI) (PLI)			
				MAX MIN MAX MIN MAX MIN			
				MT20 618 354 1667 822 2284 1656			
				PLATE PLACEMENT TOL. = 0.250 inches			
				PLATE ROTATION TOL. = 5.0 Deg.			
				JSI GRIP= 0.85 (D) (INPUT = 0.90)			
				JSI METAL= 0.18 (F) (INPUT = 1.00)			

PLATES (table in inches)									
JT TYPE	PLATES	W	LEN	Y	X				
B, C, E, F									
B TMVW+p	MT20	4.0	4.0	1.00	2.00				
D TTW+p	MT20	4.0	4.0	1.50	2.00				
H BMVW1-I	MT20	4.0	4.0						
I BMV+p	MT20	3.0	4.0						
J BVMWV-I	MT20	5.0	8.0	3.75	5.50				
K BMWVW-I	MT20	4.0	6.0						
L BVMWV-I	MT20	5.0	8.0	3.75	5.50				
M BMV+p	MT20	3.0	4.0						
N BMVWV-I	MT20	4.0	4.0						

LOADING									
TOTAL LOAD CASES: (4)									
CHORDS									
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	LC1 MAX (PLF)	MAX. UNBRAC LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	LC1 MAX (PLF)	MAX. UNBRAC LENGTH (FT)
FR-TO					FR-TO				
A-B	0/37	-84.3	-84.3	0.12 (1)	10.00	K-D	0/324	0.07 (2)	10.00
B-C	-547/0	-84.3	-84.3	0.18 (1)	6.25	K-E	-113/0	0.05 (1)	10.00
C-D	-481/0	-84.3	-84.3	0.23 (1)	6.25	C-K	-113/0	0.06 (1)	10.00
D-E	-481/0	-84.3	-84.3	0.23 (1)	6.25	N-L	-13/0	0.00 (1)	10.00
E-F	-547/0	-84.3	-84.3	0.18 (1)	6.25	B-L	0/542	0.12 (1)	10.00
F-G	0/37	-84.3	-84.3	0.12 (1)	10.00	J-H	-13/0	0.00 (1)	10.00
N-B	-815/0	0.0	0.0	0.11 (1)	7.81	J-F	0/542	0.12 (1)	10.00
H-F	-815/0	0.0	0.0	0.11 (1)	7.81				
N-M	0/11	-28.0	-28.0	0.03 (2)	10.00				
M-L	0/41	0.0	0.0	0.02 (1)	10.00				
L-C	-229/0	0.0	0.0	0.02 (1)	7.81				
L-K	0/450	-28.0	-28.0	0.24 (2)	10.00				
K-J	0/450	-28.0	-28.0	0.24 (2)	10.00				
I-J	0/41	0.0	0.0	0.02 (1)	10.00				
J-E	-229/0	0.0	0.0	0.02 (1)	7.81				
I-H	0/11	-28.0	-28.0	0.03 (2)	10.00				



DWG NO. YAM 4354217
 STRUCTURAL
 COMPONENT ONLY



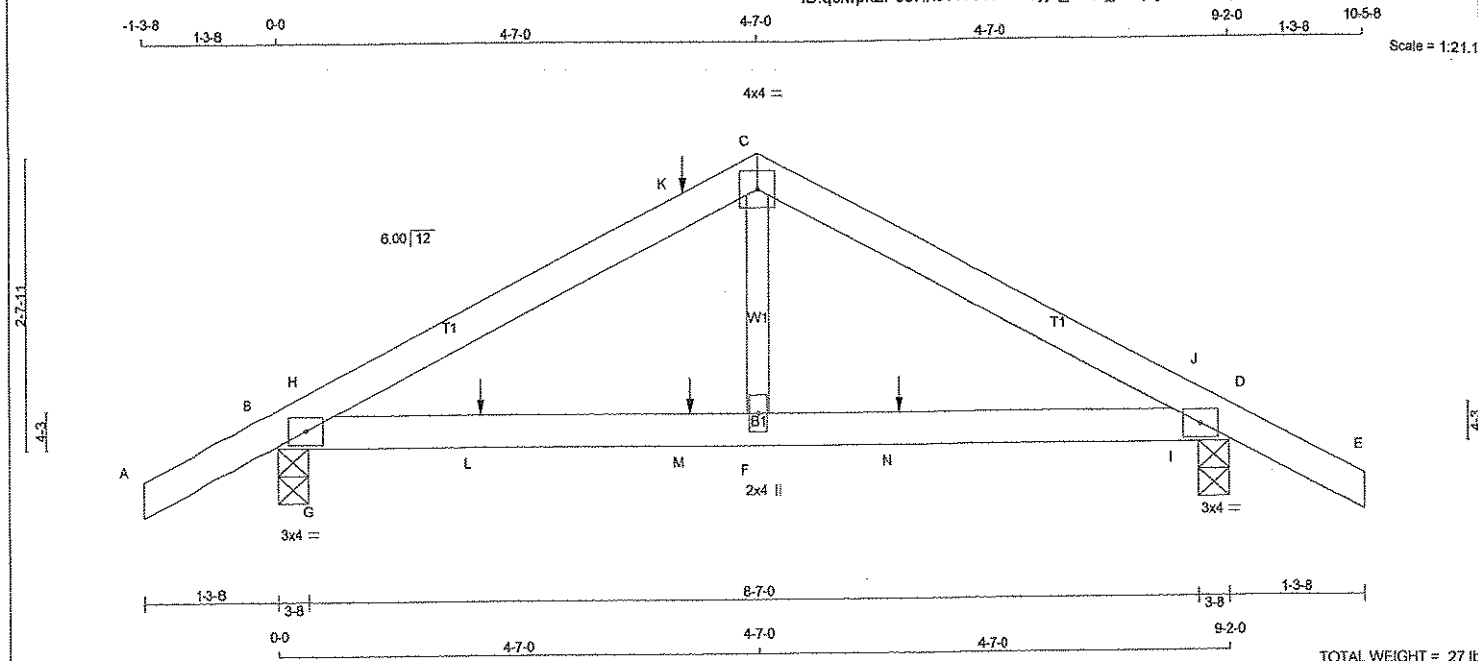
LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR.				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS						DESIGN CRITERIA					
A - C	2x4	DRY	No.2	SPF	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQ'D	SPECIFIED LOADS:						
C - E	2x4	DRY	No.2	SPF	DOWN	HORZ	UPLIFT	IN-SX	BRG	BRG	TOP CH.	LL	=	25.6	PSF
B - D	2x4	DRY	No.2	SPF	JT	VERT			IN-SX		DL	=	3.0	PSF	
					B	629	0	629	0	0	BOT CH.	LL	=	10.5	PSF
					D	629	0	629	0	0	DL	=	7.0	PSF	
ALL WEBS	2x3	DRY	No.2	SPF							TOTAL LOAD	=	46.1	PSF	
DRY: SEASONED LUMBER.															

PLATES (table is in inches)							UNFACTORED REACTIONS							SPACING = 24.0 IN. C/C		
JT	TYPE	PLATES	W	LEN	Y	X	JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS							
B	TMB1-I	MT20	3.0	4.0				COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL		
C	TTW-p	MT20	4.0	4.0			B	500	304 / 0	96 / 0	0 / 0	0 / 0	100 / 0	0 / 0		
D	TMB1-I	MT20	3.0	4.0			D	500	304 / 0	96 / 0	0 / 0	0 / 0	100 / 0	0 / 0		
F	BMW+w	MT20	2.0	4.0			BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D									
							BRACING									
							TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.									
							MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.									
							ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.									
															THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010	
															THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09 - TPIC 2011	
															(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD	

LOADING										ALLOWABLE DEFL.(LL)= L/360 (0.31")									
TOTAL LOAD CASES: (4)										CALCULATED VERT. DEFL.(LL) = L/999 (0.03")									
CHORDS										WEBS									
MAX. FACTORED					FACTORED					MAX. FACTORED									
MEMB.	FORCE	VERT. LOAD	LC1	MAX	MAX.	MEMB.	FORCE	MAX		MEMB.	FORCE	MAX							
	(LBS)	(PLF)		CSI (LC)	UNBRAC		(LBS)	CSI (LC)			(LBS)	CSI (LC)							
FR-TO		FROM	TO		LENGTH	FR-TO													
A-B	0 / 24	-84.3	-84.3	0.11 (1)	10.00	F-C	0 / 282	0.05 (2)											
B-H	-578 / 0	-84.3	-84.3	0.06 (1)	6.25	G-H	-294 / 90	0.09 (1)											
H-C	-630 / 0	-84.3	-84.3	0.21 (1)	6.25	I-J	-294 / 90	0.09 (1)											
C-J	-630 / 0	-84.3	-84.3	0.21 (1)	6.25														
J-D	-578 / 0	-84.3	-84.3	0.06 (1)	6.25														
D-E	0 / 24	-84.3	-84.3	0.11 (1)	10.00														
B-G										CSI: TC=0.21 (C-J:1), BC=0.26 (F-I:1), WB=0.06 (C-F:2), SSI=0.24 (B-G:1)									
G-F										DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10									
F-I										COMP=1.10 SHEAR=1.10 TENS= 1.10									
I-D										COMPANION LIVE LOAD FACTOR = 0.50									
F-D										TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .									
										NAIL VALUES									
										PLATE GRIP(DRY) SHEAR SECTION									



OWNER: TAMARACK
STRUCTURAL COMPONENT ONLY



LUMBER					DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER										DESIGN CRITERIA			
N. L. G. A. RULES					DESCR.	BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRO BRG	SPECIFIED LOADS:						
CHORDS	SIZE	LUMBER	SPF	VERT		HORZ	DOWN					HORZ	UPLIFT	IN-SX	TOP CH.	LL	DL	PSF
A - C	2x4	DRY	No.2	SPF														
C - E	2x4	DRY	No.2	SPF														
B - D	2x4	DRY	No.2	SPF														
ALL WEBS	2x3	DRY	No.2	SPF														
DRY: SEASONED LUMBER.																		

					<u>UNFACTORED REACTIONS</u>					<u>MAX./MIN. COMPONENT REACTIONS</u>				
					<u>1ST LCASE</u>		<u>SNOW</u>		<u>LIVE</u>	<u>PERM.LIVE</u>	<u>WIND</u>	<u>DEAD</u>	<u>SOIL</u>	
<u>PLATES (table is in inches)</u>					JT	COMBINED								
JT	TYPE	PLATES	W	LEN	Y	X	B	500	304/0	96/0	0/0	0/0	100/0	0/0
R	TMR1.1	MT20	3.0	4.0			D	500	304/0	96/0	0/0	0/0	100/0	0/0

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLETES WITH:

C	TTW-P	MT20	4.0	4.0	BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.	THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012, BCBC 2012, ABC 2014 - CSA 086-09 - TPIC 2011
D	TMB1-H	MT20	3.0	4.0		
E						
F	BMW*W	MT20	2.0	4.0		

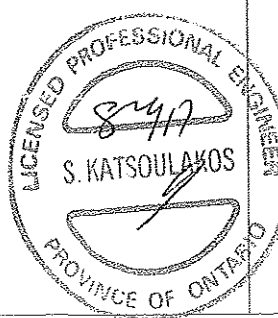
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 0.0 lbs FACTORED DOWN AT 3-11-4 ON TOP CHORD, AND AT 1-11-4, AND AT 3-11-4, AND AT 5-11-4 ON BOTTOM CHORD.	ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE EXTERNALLY REINFORCED.	NOTED
<u>LOADING</u> TOTAL LOAD CASES: (4)		ALLOWABLE DEF'L (LL)= 1/360 (0.31") CALCULATED VERT. DEF'L (LL) = 1/999 (0.03") MAXIMUM DEF'L (LL) = 260 (0.32")

DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.										ALLOWABLE DEFLECTION (1/16" JOINT) CALCULATED VERT. DEFL(TL) = 1/999 (0.04")															
CHORDS					WEBS																				
MAX. FACTORED		FACTORED		MAX. FACTORED		FACTORED		MAX. FACTORED		FACTORED		MAX. FACTORED		FACTORED											
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX. CSI (LC)	MAX. UNBRAC	MEMB.	FORCE (LBS)	MAX. CSI (LC)		MEMB.	FORCE (LBS)	MAX. CSI (LC)		CSI: TC=0.25 (C-H:1), BC=0.28 (F-G:1), WB=0.07 (C-F:2), SS=0.28 (B-G:1)											
FR-TO		FROM	TO			FR-TO				FR-TO				DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00											
A-B	0/24	-84.3	-84.3	0.12 (1)	10.00	F-C	0/282	0.07 (2)						COMPANION LIVE LOAD FACTOR = 0.50											
B-H	-578/0	-84.3	-84.3	0.09 (1)	6.25	G-H	-294/80	0.00 (1)						TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.											
H-K	-630/0	-84.3	-84.3	0.25 (1)	6.25	I-J	-294/80	0.00 (1)						NAIL VALUES											
K-C	-630/0	-84.3	-84.3	0.25 (1)	6.25									PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)											
C-J	-630/0	-84.3	-84.3	0.25 (1)	6.25									MAX MIN MAX MIN MAX MIN											
J-D	-578/0	-84.3	-84.3	0.09 (1)	6.25									MT20 618 354 1667 822 2284 1656											
D-E	0/24	-84.3	-84.3	0.12 (1)	10.00																				
B-G	0/558	-28.0	-28.0	0.28 (1)	10.00																				
G-L	0/558	-28.0	-28.0	0.28 (1)	10.00																				
L-M	0/558	-28.0	-28.0	0.28 (1)	10.00																				
M-F	0/558	-28.0	-28.0	0.28 (1)	10.00																				
F-N	0/558	-28.0	-28.0	0.28 (1)	10.00																				
N-I	0/558	-28.0	-28.0	0.28 (1)	10.00																				
I-D	0/558	-28.0	-28.0	0.28 (1)	10.00																				

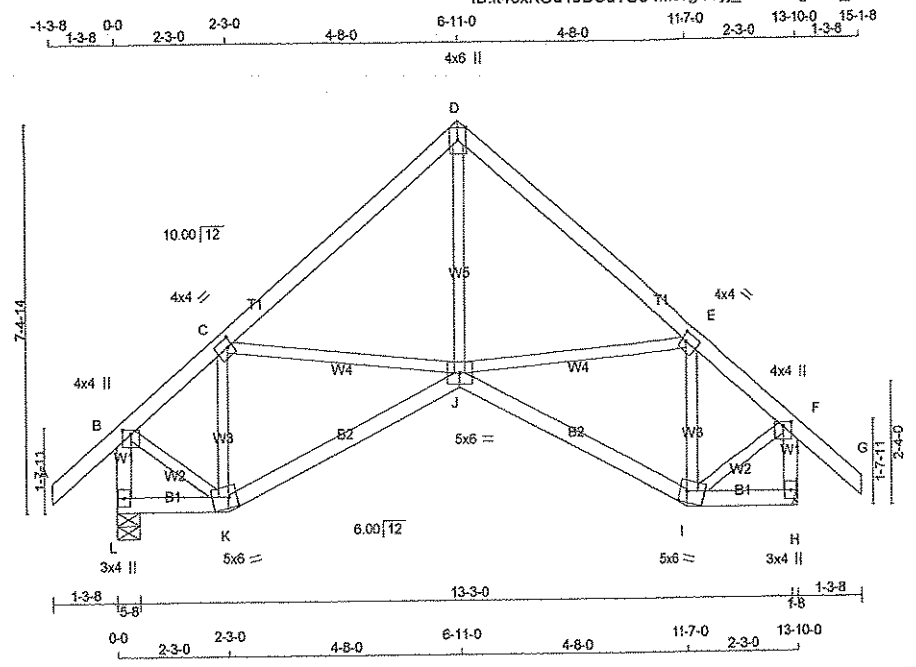
FACTORED CONCENTRATED LOADS (LBS)								PLATE PLACEMENT TOL. = 0.250 inches
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	
K	3-11-4	---	---	---	BACK	VERT	TOTAL	PLATE ROTATION TOL. = 5.0 Deg.
L	1-11-4	---	---	---	BACK	VERT	TOTAL	
M	3-11-4	---	---	---	BACK	VERT	TOTAL	JSI GRIP= 0.58 (B) (INPUT = 0.90)
N	5-11-4	---	---	---	BACK	VERT	TOTAL	JSI METAL= 0.22 (B) (INPUT = 1.00)



UNCLASSIFIED 43544
STRUCTURAL
CONTINGENT ONLY

Tamarack Roof Truss, Burlington

Scale = 1:44.4



TOTAL WEIGHT = 2 X 64 = 128 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMVW-t	MT20	4.0	4.0	2.00	1.25
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMVW-t	MT20	4.0	4.0	2.00	1.25
F	TMVW+p	MT20	4.0	4.0	1.00	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BBWW-m	MT20	5.0	6.0	2.50	2.00
J	BBWW-p	MT20	5.0	6.0		
K	BBWW-m	MT20	5.0	6.0	2.50	2.00
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
L	893	0	893	0	5-8	5-8
H	893	0	893	0	HANGER BY OTHERS	MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	717	425 / 0	145 / 0	0 / 0	0 / 0	147 / 0	0 / 0
H	717	425 / 0	145 / 0	0 / 0	0 / 0	147 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX. (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. (LC)	
FR-TO				FR-TO			
A-B	0 / 37	-84.3	-84.3 0.12 (1)	10.00	J-D	0 / 618	0.14 (1)
B-C	-627 / 0	-84.3	-84.3 0.18 (1)	6.25	J-E	0 / 65	0.01 (1)
C-D	-764 / 0	-84.3	-84.3 0.23 (1)	6.25	I-E	-457 / 0	0.09 (1)
D-E	-764 / 0	-84.3	-84.3 0.23 (1)	6.25	C-J	0 / 65	0.01 (1)
E-F	-627 / 0	-84.3	-84.3 0.18 (1)	6.25	K-C	-457 / 0	0.09 (1)
F-G	0 / 37	-84.3	-84.3 0.12 (1)	10.00	B-K	0 / 601	0.14 (1)
L-B	-862 / 0	0.0	0.0 0.09 (1)	7.81	I-F	0 / 601	0.14 (1)
H-F	-862 / 0	0.0	0.0 0.09 (1)	7.81			
L-K	0 / 0	-28.0	-28.0 0.04 (2)	10.00			
K-J	0 / 585	-28.0	-28.0 0.24 (2)	10.00			
J-I	0 / 585	-28.0	-28.0 0.24 (2)	10.00			
I-H	0 / 0	-28.0	-28.0 0.04 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = 1/360 (0.46")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.05")
ALLOWABLE DEFL.(TL) = 1/360 (0.46")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.08")

CSI: TC=0.23 (D-E:1), BC=0.24 (J-K:2), WB=0.14 (O-J:1), SSI=0.14 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

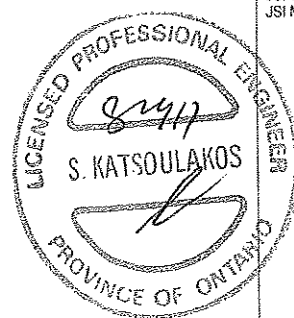
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667 622 2264 1658

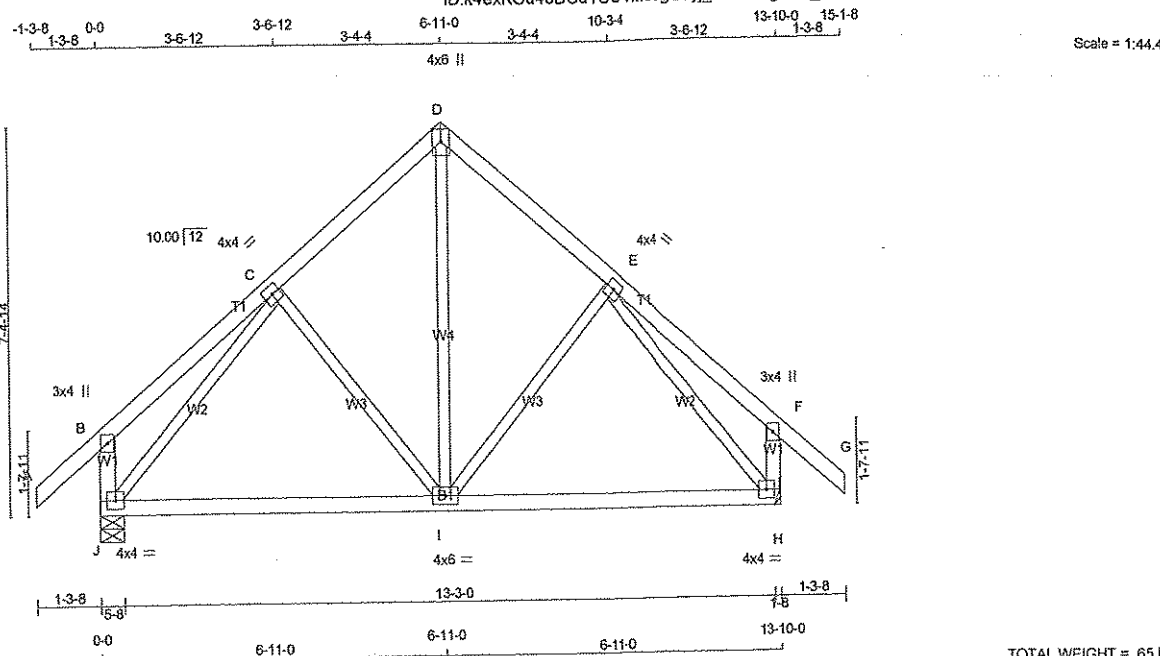
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL = 0.20 (B) (INPUT = 1.00)



DRWG NO. TAM 43547-17
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-I	MT20	4.0	4.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-I	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-I	MT20	4.0	4.0		
I	BMVW1-I	MT20	4.0	6.0		
J	BMVW1-I	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQD
JT	VERT	HORZ	DOWN	HORZ
J	893	0	893	0
H	893	0	893	0

UPLIFT IN-SX 5-8
HANGER BY OTHERS
MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	717	425 / 0	145 / 0	0 / 0	0 / 0	147 / 0	0 / 0	0 / 0
H	717	425 / 0	145 / 0	0 / 0	0 / 0	147 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 37	-84.3	-84.3 0.12 (1)	I-D	0 / 442	0.10 (1)	
B-C	0 / 21	-84.3	-84.3 0.16 (1)	E-E	-132 / 39	0.07 (1)	
C-D	-545 / 0	-84.3	-84.3 0.12 (1)	C-I	-132 / 39	0.07 (1)	
D-E	-545 / 0	-84.3	-84.3 0.12 (1)	J-C	-768 / 0	0.38 (1)	
E-F	0 / 21	-84.3	-84.3 0.16 (1)	E-H	-768 / 0	0.38 (1)	
F-G	0 / 37	-84.3	-84.3 0.12 (1)				
J-B	-228 / 0	0.0	0.0 0.02 (1)				
H-F	-228 / 0	0.0	0.0 0.02 (1)				
J-I	0 / 487	-28.0	-28.0 0.44 (2)				
I-H	0 / 487	-28.0	-28.0 0.44 (2)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.48")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (0.46")
CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CSI: TC=0.16 (E-F:1), BC=0.44 (I-J:2), WB=0.36 (C-J:1), SSI=0.16 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

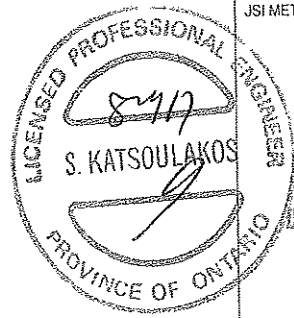
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	
MT20	618 354	1667 822	2284 1656

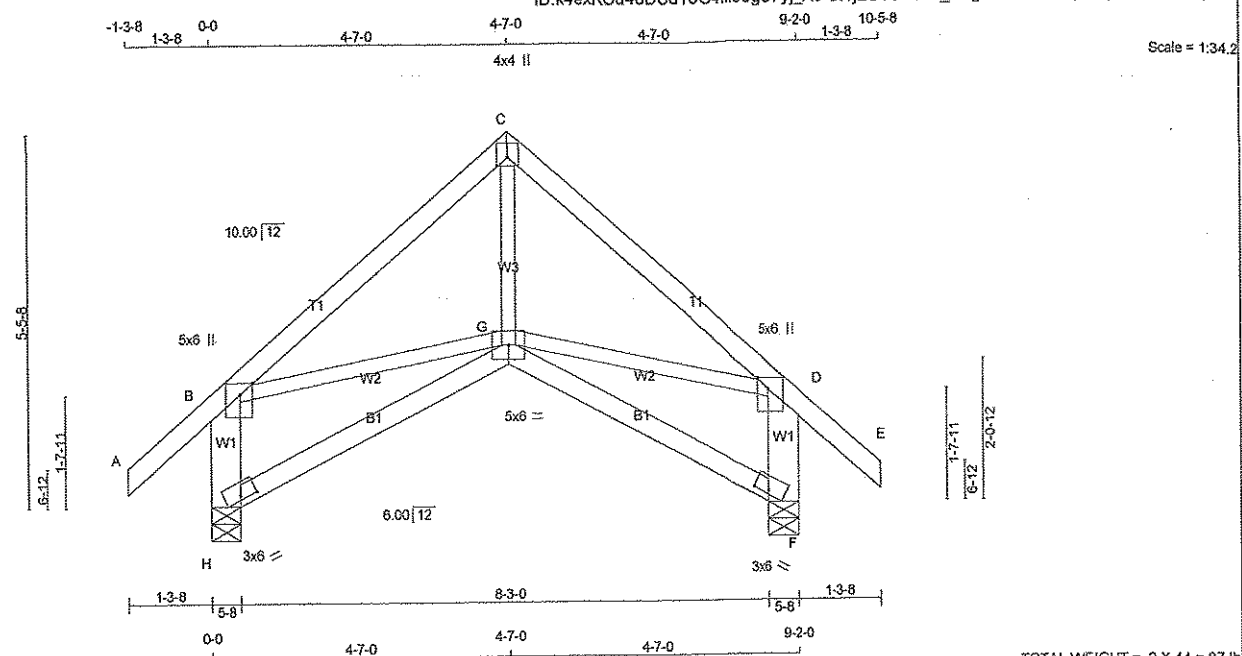
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (E) (INPUT = 0.90)
JSI METAL= 0.26 (E) (INPUT = 1.00)



DRWG NO. TAM 43548-17
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 2 X 44 = 87 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - E	2x4	DRY No.2	SPF
H - B	2x6	DRY No.2	SPF
F - D	2x6	DRY No.2	SPF
H - G	2x4	DRY No.2	SPF
G - F	2x4	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TTW+p	MT20	4.0	4.0	1.50	2.00
D	TMVW+p	MT20	5.0	6.0	2.00	2.25
F	BVM1-I	MT20	3.0	6.0	0.75	3.00
G	BBWWW+p	MT20	5.0	6.0		
H	BVM1-I	MT20	3.0	6.0	0.75	3.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
H	631	0	631	0	5-8	5-8
F	631	0	631	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS				
		SNOW	LIVE	PERM. LIVE	WIND	DEAD
H	502	305 / 0	96 / 0	0 / 0	0 / 0	100 / 0
F	502	305 / 0	96 / 0	0 / 0	0 / 0	100 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1		MAX. FACTORED VERT. LOAD LC1 (PLF)	MAX. FACTORED VERT. LOAD LC1 (LC)	MAX. FACTORED VERT. LOAD LC1 UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD LC1 (LC)
		FROM	TO						
FR-TO									
A-B	0 / 37	-84.3	-84.3	0.12 (1)	10.00		G-C	0 / 320	0.07 (2)
B-C	-504 / 0	-84.3	-84.3	0.23 (1)	6.25		B-G	0 / 393	0.09 (1)
C-D	-504 / 0	-84.3	-84.3	0.23 (1)	6.25		G-D	0 / 393	0.09 (1)
D-E	0 / 37	-84.3	-84.3	0.12 (1)	10.00				
H-B	-587 / 0	0.0	0.0	0.04 (1)	7.81				
F-D	-587 / 0	0.0	0.0	0.04 (1)	7.81				
H-G	0 / 0	-28.0	-28.0	0.18 (3)	10.00				
G-F	0 / 0	-28.0	-28.0	0.18 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 3.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.31")

CALCULATED VERT. DEFL.(LL) = L/999 (0.04")

ALLOWABLE DEFL.(TL) = L/360 (0.31")

CALCULATED VERT. DEFL.(TL) = L/999 (0.06")

CSI: TC=0.23 (C-D:1), BC=0.18 (F-G:3), WB=0.09 (B-G:1), SSI=0.12 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1697

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (C) (INPUT = 0.90)

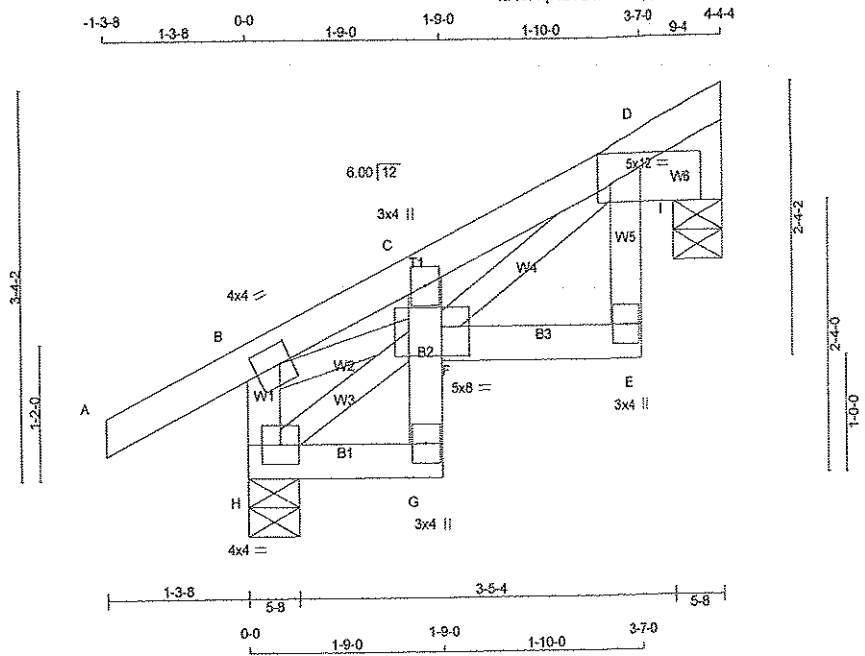
JSI METAL= 0.12 (D) (INPUT = 1.00)



DWG NO. TAM 43549 17

STRUCTURAL

COMPONENT ONLY



TOTAL WEIGHT = 5 X 21 = 107 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
E - D	2x4	DRY	No.2
H - B	2x4	DRY	No.2
H - G	2x4	DRY	No.2
G - C	2x4	DRY	No.2
F - E	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-t	MT20	4.0	4.0	2.00 1.25
C	TMV+p	MT20	3.0	4.0	
D	TMVWW1-p	MT20	5.0	12.0	Edge 1.50
E	BMV+p	MT20	3.0	4.0	
F	BVMWW1	MT20	5.0	8.0	3.00 3.00
G	BMV+p	MT20	3.0	4.0	
H	BMVW1	MT20	4.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	IN-SX
J(D)	131	0	156	0	5-8
H	361	0	361	0	5-8

UNFACTORED REACTIONS		1ST LCASE	MAX/MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
J(D)	131	68 / 0	33 / 0
H	277	185 / 0	43 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J(D), H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	CS1 (LC)	MEMB.	FORCE (LBS)
FR-TO		FROM	TO	FR-TO	
A-B	0 / 26	-84.3	-84.3 0.11 (1)	F-D	0 / 255
B-C	-275 / 0	-84.3	-84.3 0.11 (1)	H-F	-25 / 0
C-D	-275 / 0	-84.3	-84.3 0.06 (1)	B-F	0 / 226
E-I	0 / 48	0.0	0.0 0.09 (1)	D-J	-240 / 0
I-D	0 / 48	0.0	0.0 0.09 (1)	I-J	0 / 183
H-B	-325 / 0	0.0	0.0 0.03 (1)		
H-G	0 / 21	-28.0	-28.0 0.02 (2)		
G-F	0 / 39	0.0	0.0 0.03 (1)		
F-C	-103 / 0	0.0	0.0 0.03 (1)		
F-E	0 / 51	-28.0	-28.0 0.04 (2)		

DESIGN CRITERIA

SPECIFIED LOADS:	
TOP CH.	LL = 25.6 PSF
	DL = 3.0 PSF
BOT CH.	LL = 10.5 PSF
	DL = 7.0 PSF
TOTAL LOAD	= 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CS1: TC=0.11 (A-B:1), BC=0.04 (E-F:2), WB=0.05 (D-F:1), SS=0.08 (B-C:1)

COL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

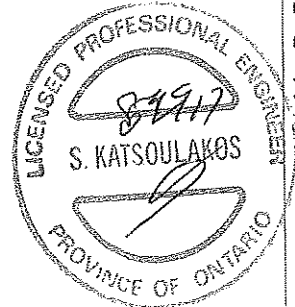
PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

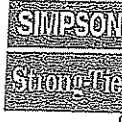
JSI GRIP= 0.66 (D) (INPUT = 0.90)

JSI METAL= 0.13 (B) (INPUT = 1.00)



BY: T. TAN 43534/17
STRUCTURAL
COMPONENT ONLY

LUS - Double Shear Joist Hangers



All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

MATERIAL: 18 gauge

FINISH: G90 galvanized

DESIGN:

- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

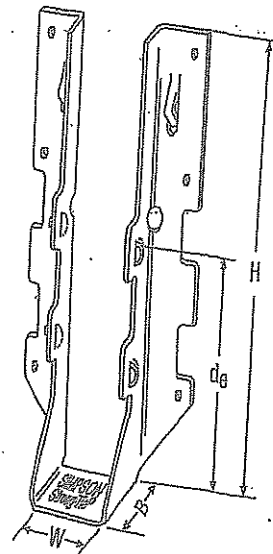
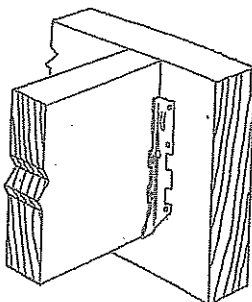
INSTALLATION:

- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/4" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

- These hangers cannot be modified.

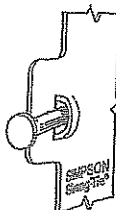
Typical LUS Installation



LUS28

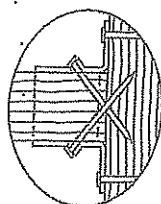
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
		W	H	B	dg ¹	Face	Joist	D-Flt-L		S-P-F	
								Uplift (K _u =1.15)	Normal (K _n =1.00)	Uplift (K _u =1.15)	Normal (K _n =1.00)
LUS24	18	1 1/16	3 1/4	1 3/4	1 1/16	4-10d	2-10d	710	1630	645	1155
LUS24-2	18	3 1/4	3 1/4	2	1 1/16	4-16d	2-16d	835	2020	590	1485
LUS26	18	1 1/16	4 1/4	1 3/4	3/8	4-10d	4-10d	1420	2170	1290	1630
LUS26-2	18	3 1/4	4 1/4	2	4	4-16d	4-16d	1720	2595	1545	1920
LUS26-3	18	4 1/4	4 1/4	2	3/4	4-16d	4-16d	1720	2595	1545	2340
LUS28	18	1 1/16	6 1/4	1 3/4	3/8	6-10d	4-10d	1420	2520	1290	1790
LUS28-2	18	3 1/4	7	2	4	6-16d	4-16d	1720	3325	1545	2676
LUS28-3	18	4 1/4	6 1/4	2	3/4	6-16d	4-16d	1720	3325	1545	2375
LUS210	18	1 1/16	7 1/4	1 3/4	3/8	8-10d	4-10d	1420	2785	1290	2210
LUS210-2	18	3 1/4	9	2	6	8-16d	6-16d	2580	4500	2320	3105
LUS210-3	18	4 1/4	8 1/4	2	5 1/4	8-16d	6-16d	2580	3345	2320	2375

1. dg is the distance from the seat of the hanger to the highest joist nail.

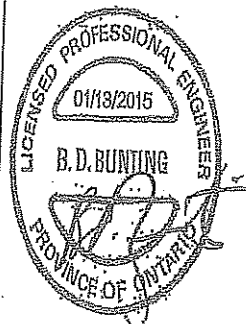


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Top View.



This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2015. This information is updated periodically and should not be relied upon after December 31, 2016. Contact Simpson Strong-Tie for current information and limited warranty or see www.strongtie.com.

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1-SPEC LUS 15 4/15 exp. 12/16

800-899-6099

www.strongtie.com

HUS/LJS - Double Shear Joist Hangers



All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: See table

FINISH: G90 galvanized

DESIGN:

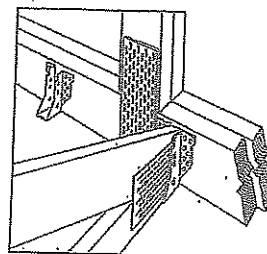
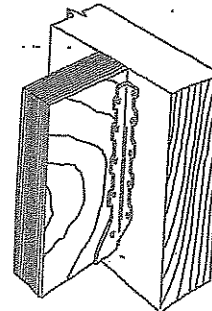
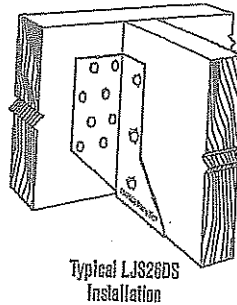
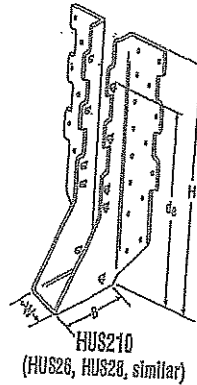
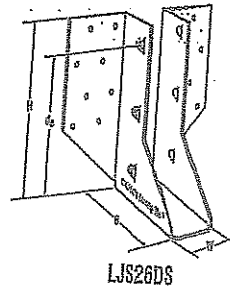
- Factored resistances are in accordance with CSA D86-14
- Uplift resistances have been increased 15%. No further increase is permitted
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

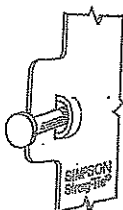
- See current catalogue for options



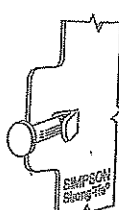
Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

Model No.	Ga	Dimensions (in)		Fasteners		Factored Resistance (lbs)			
						D-Fir-L		S-P-F	
		W	H	B	d _g ¹	Face	Joist	Uplift (K _u =1.15)	Normal (K _n =1.00)
LJS26DS	18	1 1/16	5	3 1/2	4 3/4	16-16d	6-16d	2055	4265
HUS26	18	1 1/8	5 1/2	3	3 1/4	14-16d	6-16d	2705	4940
HUS28	18	1 1/8	7 1/2	3	6 1/2	22-16d	8-16d	3605	5365
HUS210	18	1 1/8	9 1/2	3	7 1/2	30-16d	10-16d	4505	5795
HUS1.81/10	18	1 1/16	9	3	8	30-16d	10-16d	4505	6450

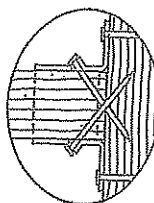
1. d_g is the distance from the seat of the hanger to the highest joist nail.



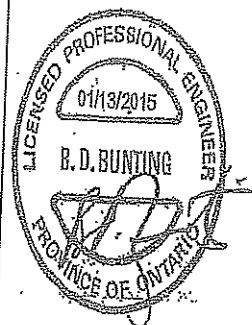
Dome Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,560



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



U.S. PATENT
DESIGN

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SPEC-HUS16 1/16 x 0.32/16

800-999-5099

www.simpsonstrongtie.com

HGUS - Double Shear Joist Hangers

SIMPSON

Strong-Tie

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: 12 gauge

FINISH: G90 galvanized

DESIGN:

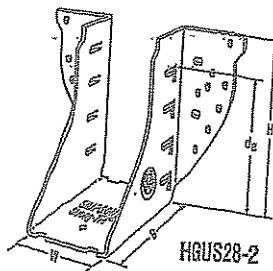
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

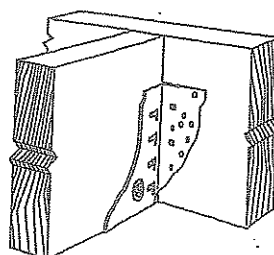
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

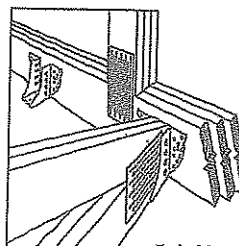
- See current catalogue for options.



HGUS28-2



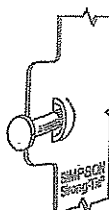
Typical HGUS Installation



Typical HGUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

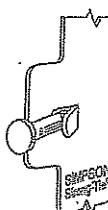
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
		W	H	B	d _g	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _u =1.15)	Normal (K _n =1.00)	Uplift (K _u =1.15)	Normal (K _n =1.00)
HGUS26	12	1 1/2	5 1/2	6	4 1/2	20-16d	8-16d	2685	6625	2685	6700
HGUS26-2	12	3 1/2	5 1/2	4	4 1/2	20-16d	8-16d	4385	8950	3100	6355
HGUS26-3	12	4 1/2	5 1/2	4	4 1/2	20-16d	8-16d	4385	8950	3100	6355
HGUS26-4	12	6 1/2	5 1/2	4	4 1/2	20-16d	8-16d	4385	8950	3100	6355
HGUS28	12	1 1/2	7 1/2	5	6 1/2	36-16d	12-16d	3310	7675	3100	6900
HGUS28-2	12	3 1/2	7 1/2	4	6 1/2	36-16d	12-16d	6070	12980	4310	9215
HGUS28-3	12	4 1/2	7 1/2	4	6 1/2	36-16d	12-16d	6070	12980	4310	9215
HGUS28-4	12	6 1/2	7 1/2	4	6 1/2	36-16d	12-16d	6070	12980	4310	9215
HGUS210-2	12	3 1/2	9 1/2	4	8 1/2	48-16d	16-16d	6840	14645	4855	10400
HGUS210-3	12	4 1/2	9 1/2	4	8 1/2	48-16d	16-16d	6840	14645	4855	10400
HGUS210-4	12	6 1/2	9 1/2	4	8 1/2	48-16d	16-16d	6840	14645	4855	10400
HGUS212-4	12	6 1/2	10 1/2	4	10 1/2	56-16d	20-16d	7040	14995	5425	10845
HGUS214-4	12	6 1/2	12 1/2	4	11 1/2	66-16d	22-16d	10130	16400	7195	11645

1. d_g is the distance from the seat of the hanger to the highest joist nail.

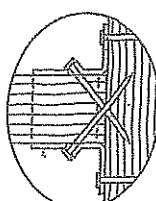


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

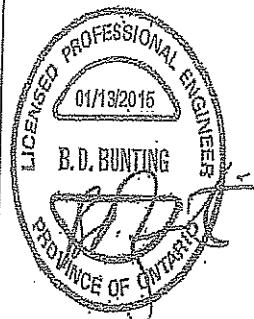
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



UNIT STATES DESIGN

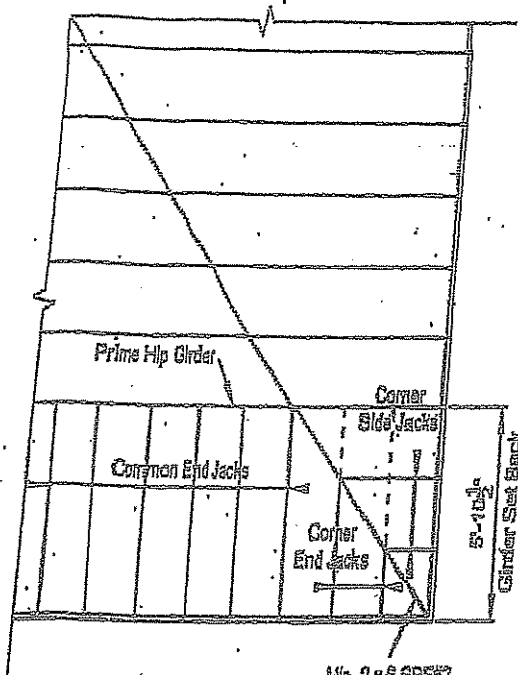
This technical bulletin is effective until December 31, 2016, and no new information is available as of that date. This information is for general informational purposes only and should not be relied upon after December 31, 2016. For more information on Simpson Strong-Tie products and services, visit www.simpsonstrongtie.com or call 800-999-6099.

800-999-6099
www.simpsonstrongtie.com

MICRO CITY

ENGINEERING SERVICES INC.

TEL: (519) 287-2242



45° Hip End

LUMBER SPECIFICATION

TOP CHORD : 2x4 SPF#2

BOTTOM CHORD : 2x4 SPF#2

WEBS : 2x3 SPF#2

UNLESS OTHERWISE SHOWN

DESIGN LOAD:

TOP CHORD LIVE LOAD : 39.4 P.S.F.

TOP CHORD DEAD LOAD : 3.0 P.S.F.

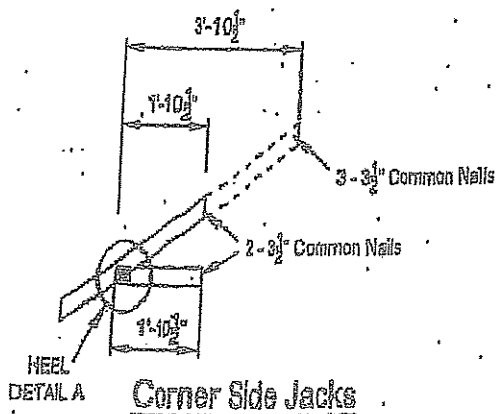
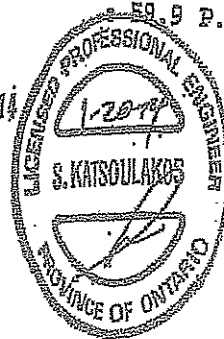
BOTTOM CHORD LIVE LOAD : 10.5 P.S.F.

BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

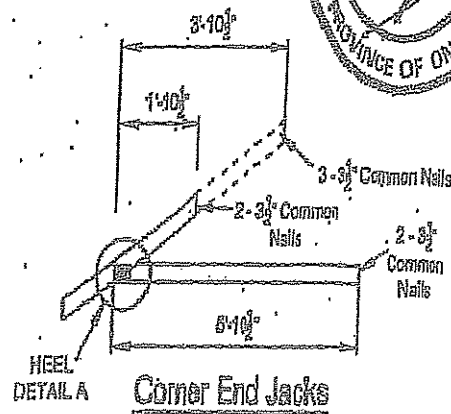
TOTAL LOAD : 59.9 P.S.F.

DWG NO. T11 2495.11

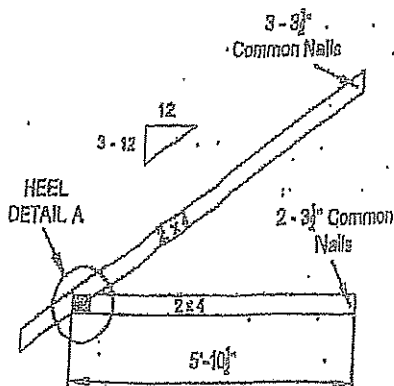
STRUCTURAL
COMPONENT ONLY



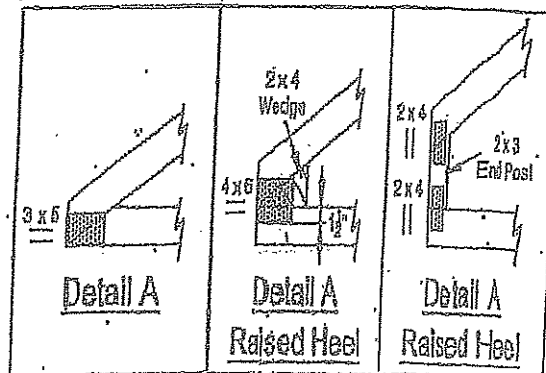
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

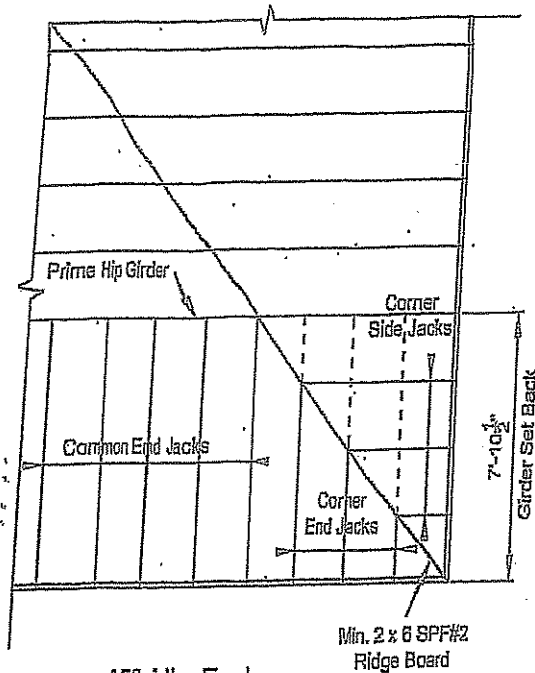
LUMBER SPECIFICATION

TOP CHORD : 2x4 SPF#2
 BOTTOM CHORD : 2x4 SPF#2
 WEBS : 2x3 SPF#2
 UNLESS OTHERWISE SHOWN

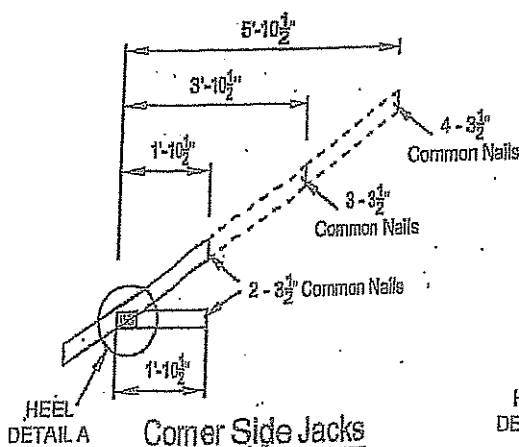
DESIGN LOAD:

TOP CHORD LIVE LOAD : 34.8 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

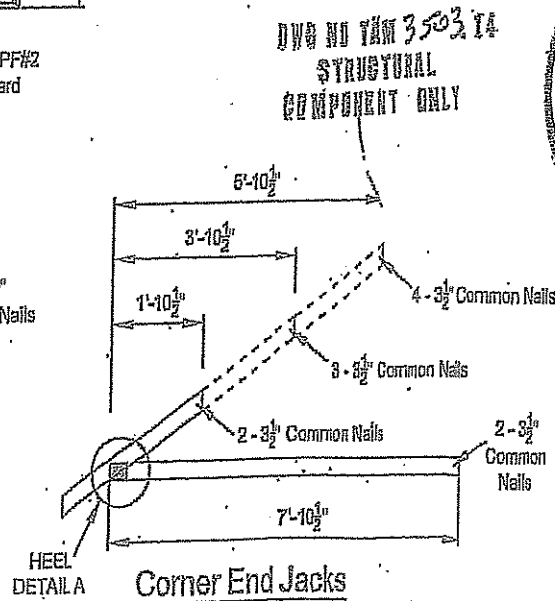
TOTAL LOAD : 44.8 P.S.F.



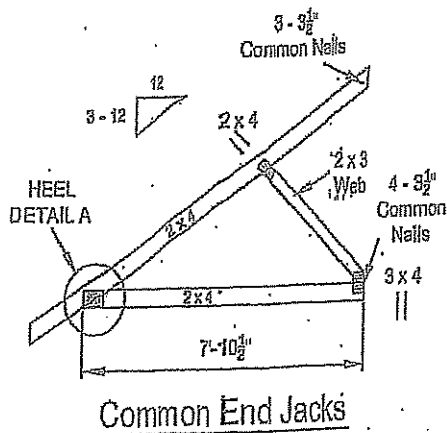
45° Hip End



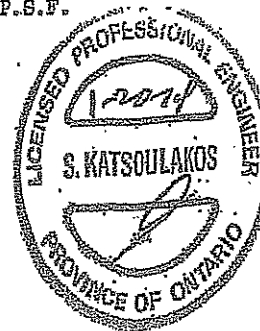
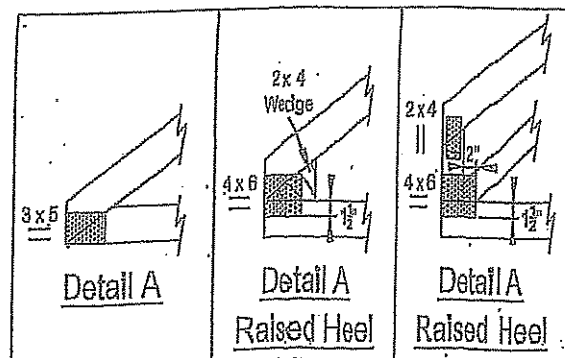
Corner Side Jacks



Corner End Jacks



Common End Jacks

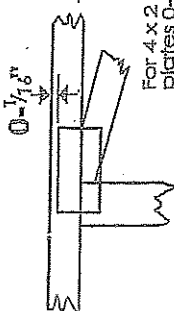


NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
 (TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

Symbols

PLATE LOCATION AND ORIENTATION

Center plate on joint unless x, y offsets are indicated. Dimensions are in 1/16-in. increments or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/8\" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*Plate location details available in Mitek software or upon request.

PLATE SIZE

4 x 4

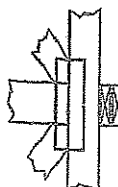
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator Bracing if indicated.

BEARING



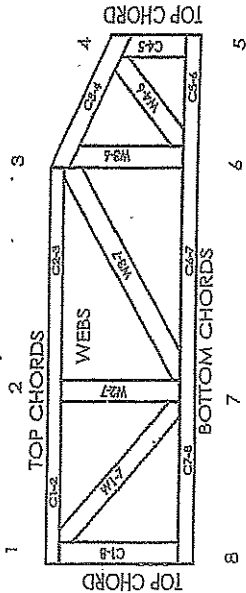
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing
BCSI: Building Component Safety Information, Guide to Good Practices for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in 1/16-in. increments or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FAREST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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Mitek
POWER TO PERFORM™

Mitek Engineering Reference Sheet, M11-7473C rev. 10-08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g., diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing. See alternative T.1.1 or Eliminator Bracing should be considered.
3. Never exceed the design loadings shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Nails and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

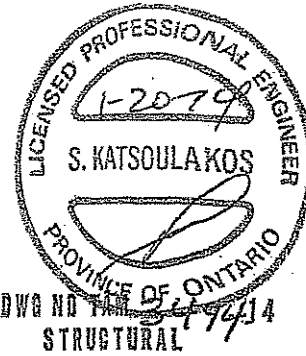
Micro City Engineering Services Inc.
(BCIN: 26064; FIRM BCIN: 29991)

RR #1, Po Box 61

Glencoe, Ontario

NOL 1M0

(519) 287 - 2242; Fax: (519) 287 - 5750 (Call)



Responsibilities:

Micro City Engineering Services is responsible for the design of trusses as individual components.

It is the responsibilities of others to ascertain that the design loads utilized on this (these) drawing(s) meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdiction over such decisions.

All dimensions are to be verified by the owner, contractor, architect, or other authority having input over such decisions prior to truss component manufacture. At no time shall Micro City Engineering Services Inc. or its employees be responsible for dimension errors.

Micro City Engineering Services Inc. bears no responsibility for the erection of any truss components. Persons erecting truss components are cautioned to seek professional advice regarding temporary and permanent bracing systems and to be totally familiar with all aspects of truss erection prior to proceeding on any truss component erection job. Any bracing shown on Micro City Engineering Services Inc. or Tamarack Roof Trusses Inc. sealed or unsealed truss component drawings is specified for the single truss component in question and is identified as an integral part of the design for that particular truss component but is not meant to represent the only required bracing for that particular truss component when installed as a component in a series of truss components in a roof truss system.

~~It is the truss manufacturer's responsibility to ensure that trusses are manufactured in accordance with Micro City Engineering Services Inc. specifications outlined below:~~

SPECIFICATIONS:

Truss components sealed by Micro City Engineering Services Inc. must conform to the relevant sections of the current Building Code of Ontario and Canada (Part 4 or Part 9) or the current Farm Building Code of Canada in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the Truss Plate Institute of Canada (TPIC). All unit lumber and nailing stresses identified on truss component design drawings and/or used in the design of individual truss components shall conform to the current CSA Wood Design standard identified in the current Building Code and TPIC Design Standards.

The lumber used to manufacture any truss component is to conform to the specified size and grade identified on the truss drawing.

The lumber used in the manufacture of any truss component is not to exceed 19% during its service use unless specifically noted on the truss drawing.

The lumber used in the manufacture of any truss component is not to be treated with any chemicals during its service life unless specifically noted on the truss drawing.

Connector plates shall be applied to both faces of the truss component at each joint and shall be positioned exactly as specified.

The top chord of any truss component is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the sealed truss component drawing but not exceeding 24" o/c (Part 9 design) and not exceeding 48" o/c (Part 4 or Agricultural design).

When a truss component is to be installed with no rigid ceiling attached directly to the bottom chord, then the bottom chord is to be laterally braced at intervals not exceeding 3m (or 10'-0").

All sealed or unsealed truss component drawings provided by Micro City Engineering Services Inc. Or Tamarack Roof Trusses Inc. should be read in conjunction with the following:

Warning-Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473C rev 10-'08 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer - not the truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpica.com and BCSI Building Component Safety Information available from the Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA, 22314.