

2-06-00

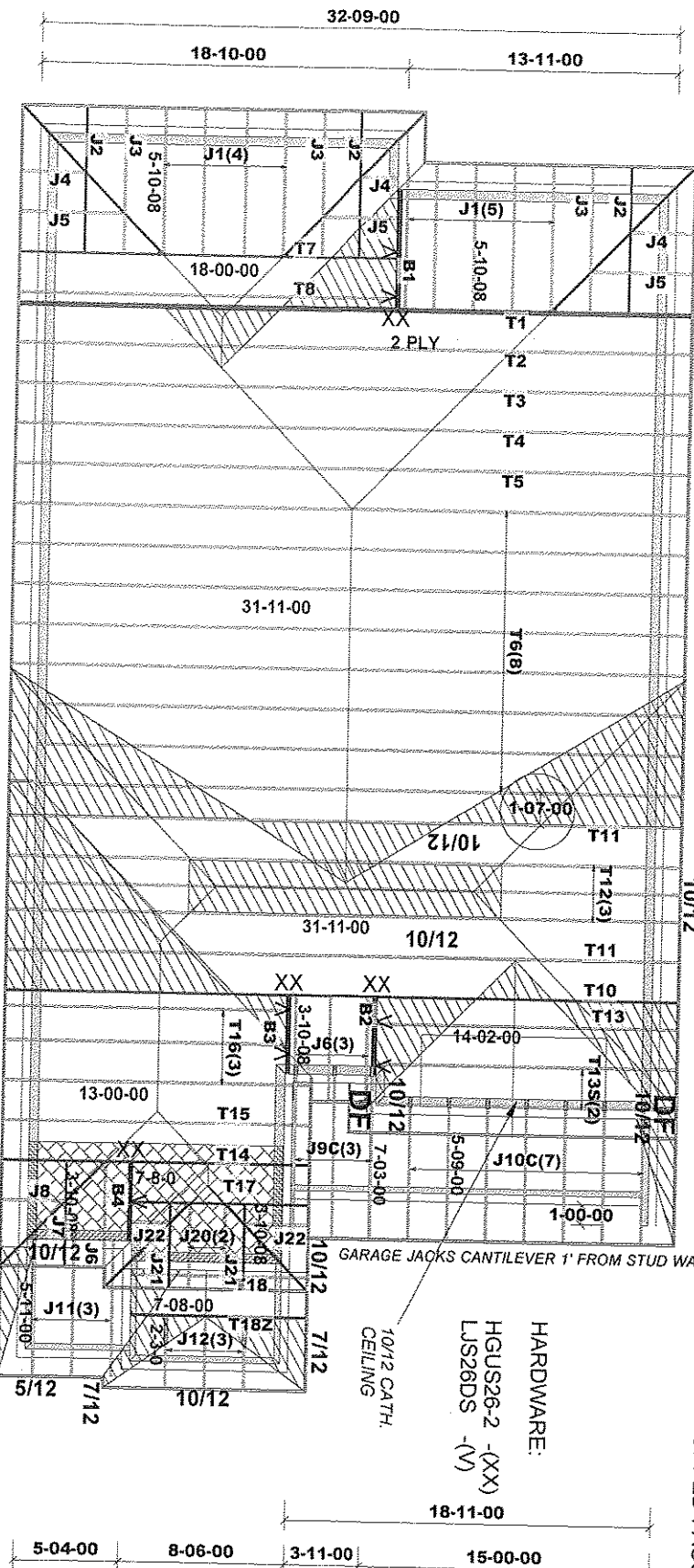
46-06-00

50-10-00

8-00-00

3-08-00

DF: 1' 0" DROPPED FASCIA ONLY



HARDWARE:

HGUS26-2 -(XX)
LJS26DS -(V)

10/12 CATH.
CEILING

GARAGE JACKS CANTILEVER 1' FROM STUD WALL

56-00-00

1' RAISED FASCIA 10/12

1' 0" RAISED PLATE

1' 0" RAISED CEILING

B: BARREL VAULT CEILING
(C.R.F.)

BEAMS:
ALL BEAMS (B) 2 - 2X10 SPF #2

ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. 2012 ROOF RAFTERS THAT CROSS OVER OR MEET TRUSSES TO BE 2X4 SPF #2 24" O.C. WITH A VERT POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DIST. BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

CONVENTIONAL FRAMING

ALL ROOF SLOPES ARE 6/12 UNLESS OTHERWISE NOTED

T - 170391

ASPHALT SHINGLES
12" FINISHED O.H.
5" CLADDING ALLOWANCE
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD
HEEL: R.T.M.C.

11/0/98

TAMARACK TAMARACK ROOF TRUSSES INC.		Job Track: 43954 Layout ID: 282771 Plan Log: 92601	Builder / Location: GREENPARK HOMES / WATERDOWN	Model / Elevation: ROSEWOOD 6 / 1
Project: RUSSEL GARDENS PHASE 2	Date: 8/29/2017 Designer: colin/s.v.e.	THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.		

T - 170391

ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. 2012 ROOF RAFTERS THAT CROSS OVER OR MEET TRUSSES TO BE 2X4 SPF #2 24" O.C. WITH A VERT POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DIST. BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

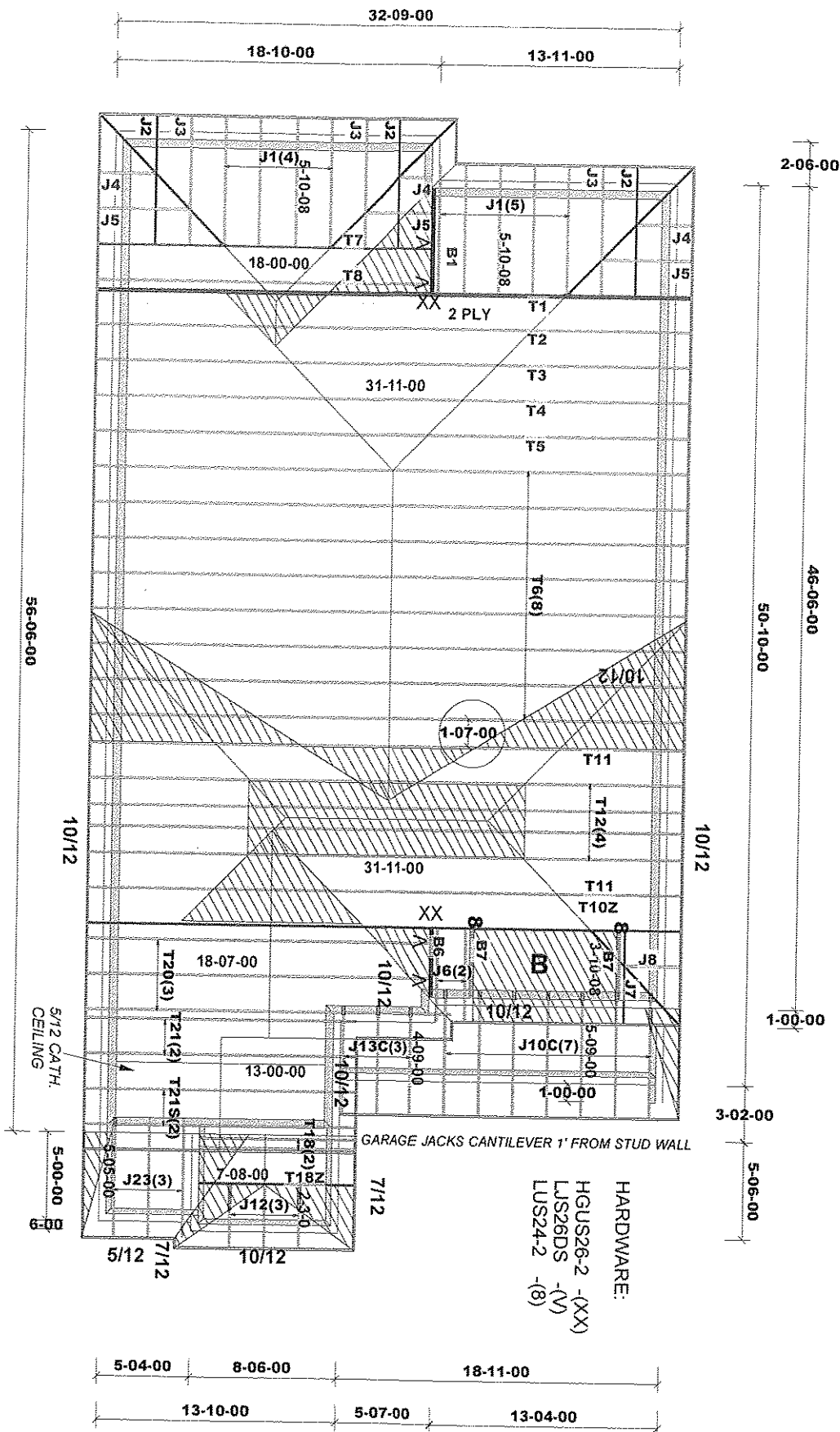
**DENOTES:
CONVENTIONAL
FRAMING**

ALL ROOF SLOPES ARE 6/12
UNLESS OTHERWISE NOTED

B: BARREL VAULT CEILING
(C.R.F.)

BEAMS:
ALL BEAMS (B) 2 - 2X10 SPF #2

HARDWARE:
HGUS26-2 -(XX)
LJS26DS -(V)
LUS24-2 -(8)



Job Track: **43954**

Layout ID: 282772

Plan Log: **92601**

Builder / Location

GREENPARK HOMES / WATERDOWN

Model / Elevation:

ROSEWOOD 6 / 2

Project: **RUSSEL GARDENS PHASE 2**
 Date: **8/29/2017** Designer: **colin/s.v.e**

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.



Delivery Shiplist

DATE	08/29/17
SALES REP	Mario

JOB TRACK:43954 LAYOUT ID: 282771 LOCATION: WATERDOWN
 BUILDER: GREENPARK - RUSSELL GARDENS PH 2 SUB-BUILDER:
 MODEL: ROSEWOOD 6 ELEVATION: 1

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	T1 HIP GIRDER	6.00	31-11-00	04-01-04	2 X 4	2 X 6	01-03-08	01-02-00	317.54		
	2 Ply		0.00							194.68		
	1	T2 HIP	6.00	31-11-00	05-01-04	2 X 4	2 X 4	01-03-08	01-02-00	127.46		
			0.00					01-03-08	01-02-00	79.83		
	1	T3 HIP	6.00	31-11-00	06-01-04	2 X 4	2 X 4	01-03-08	01-02-00	130.07		
			0.00					01-03-08	01-02-00	81.34		
	1	T4 HIP	6.00	31-11-00	07-01-04	2 X 4	2 X 4	01-03-08	01-02-00	131.31		
			0.00					01-03-08	01-02-00	81.33		
	1	T5 HIP	6.00	31-11-00	08-01-04	2 X 4	2 X 4	01-03-08	01-02-00	132.23		
			0.00					01-03-08	01-02-00	83.17		
	8	T6 COMMON	6.00	31-11-00	09-01-12	2 X 4	2 X 4	01-03-08	01-02-00	1041.60		
			0.00					01-03-08	01-02-00	644.00		
	1	T10 HIP GIRDER	10.00	31-11-00	04-10-07	2 X 6	2 X 6	01-03-08	01-07-11	191.62		
			0.00					01-03-08	01-07-11	113.34		
	2	T11 HIP	10.00	31-11-00	06-06-07	2 X 4	2 X 4	01-03-08	01-07-11	271.96		
			0.00					01-03-08	01-07-11	168.34		
	3	T12 HIP	10.00	31-11-00	08-02-07	2 X 4	2 X 4	01-03-08	01-07-11	459.33		
			0.00					01-03-08	01-07-11	291.00		
	1	T7 HIP GIRDER	6.00	18-00-00	04-01-04	2 X 4	2 X 6	01-03-08	01-02-00	81.74		
			0.00					01-03-08	01-02-00	52.67		
	1	T8 HIP	6.00	18-00-00	05-01-04	2 X 4	2 X 4	01-03-08	01-02-00	75.17		
			0.00					01-03-08	01-02-00	48.50		
	1	T13 COMMON	10.00	14-02-00	06-06-08	2 X 4	2 X 4	01-03-08	00-07-11	57.15		
			0.00					01-03-08	00-07-11	36.67		
	2	T13S ROOF	10.00	14-02-00	06-06-08	2 X 4	2 X 4	01-03-08	00-07-11	123.90		
			10.00					01-03-08	00-07-11	82.00		
	1	T14 HIP GIRDER	10.00	13-00-00	04-10-07	2 X 4	2 X 6	01-03-08	01-07-11	68.14		
			0.00					01-03-08	01-07-11	44.00		
	1	T15 HIP	10.00	13-00-00	07-06-07	2 X 4	2 X 4	01-03-08	02-07-11	69.94		
			0.00					01-03-08	02-07-11	44.83		
	3	T16 COMMON	10.00	13-00-00	08-00-11	2 X 4	2 X 4	01-03-08	02-07-11	203.58		
			0.00					01-03-08	02-07-11	129.99		
	1	T17 HIP GIRDER	10.00	07-08-00	04-00-07	2 X 4	2 X 4	01-03-08	01-07-11	42.64		
			0.00					01-03-08	01-07-11	28.33		
	1	T18 COMMON	7.00	07-08-00	02-09-11	2 X 4	2 X 4	01-03-08	00-06-14	25.58		
			0.00					01-03-08	00-06-14	17.50		



Delivery Shiplist

DATE	08/29/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 282771 LOCATION: WATERDOWN
 BUILDER: GREENPARK - RUSSELL GARDENS PH SUB-BUILDER:
 MODEL: ROSEWOOD 6 ELEVATION: 1

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY PLY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER TOP BOT	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	1	T18Z COMMON	7.00 0.00	07-08-00	02-09-11	2 X 4 2 X 4	01-03-08 01-03-08	00-06-14 00-06-14	25.58 17.50		
	9	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4 2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	151.11 96.03		
	3	J2 JACK-OPEN	6.00 0.00	05-10-08	02-00-12	2 X 4 2 X 4	01-03-08 -04-01-01	01-02-00 00-03-08	48.48 30.99		
	3	J3 JACK-OPEN	6.00 0.00	05-10-08	03-00-12	2 X 4 2 X 4	01-03-08 -02-01-01	01-02-00 00-03-08	56.13 35.01		
	3	J4 JACK-OPEN	6.00 0.00	01-10-08	02-00-12	2 X 4 2 X 4	01-03-08 -00-01-01	01-02-00 00-03-08	25.29 17.01		
	3	J5 JACK-OPEN	6.00 0.00	01-10-08	03-00-12	2 X 4 2 X 4	01-03-08 01-10-15	01-02-00 00-04-11	27.93 17.49		
	4	J6 JACK-OPEN	10.00 0.00	03-10-08	04-10-07	2 X 4 2 X 4	01-03-08 00-00-00	01-07-11 04-10-07	55.00 34.68		
	1	J7 JACK-OPEN	10.00 0.00	03-10-08	03-01-09	2 X 4 2 X 4	01-03-08 -02-01-01	01-07-11 00-03-08	13.73 9.33		
	1	J8 JACK-OPEN	10.00 0.00	01-10-08	03-01-09	2 X 4 2 X 4	01-03-08 -00-01-01	01-07-11 00-03-08	9.99 7.00		
	3	J9C JACK-OPEN	6.00 0.00	07-03-00	03-11-11	2 X 4 2 X 4	01-03-08 00-00-00	00-04-03 03-11-11	72.18 45.00		
	7	J10C JACK-OPEN	6.00 0.00	05-09-00	03-02-11	2 X 4 2 X 4	01-03-08 00-00-00	00-04-03 03-02-11	121.45 77.00		
	3	J11 JACK-OPEN	5.00 0.00	05-11-00	02-09-10	2 X 4 2 X 4	01-03-08 00-00-00	00-04-01 02-09-10	47.37 27.99		
	3	J12 JACK-OPEN	10.00 0.00	02-03-00	02-09-10	2 X 4 2 X 4	01-03-08 00-00-00	00-11-02 02-09-10	26.01 18.00		
	2	J20 JACK-OPEN	10.00 0.00	02-10-08	04-00-07	2 X 4 2 X 4	01-03-08 00-00-00	01-07-11 04-00-07	22.24 14.66		
	2	J21 JACK-OPEN	10.00 0.00	02-10-08	03-01-09	2 X 4 2 X 4	01-03-08 -01-01-01	01-07-11 00-03-08	23.66 16.34		
	2	J22 JACK-OPEN	10.00 0.00	01-10-08	03-01-09	2 X 4 2 X 4	01-03-08 -00-01-01	01-07-11 00-03-08	15.82 11.34		

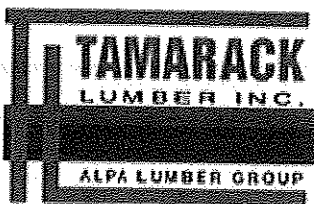
TOTAL # TRUSS= 82.00

TOTAL BFT OF ALL TRUSSES=

2696.89 BFT. TOTAL WEIGHT OF ALL TRUSSES= 4292.93 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
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Delivery Shiplist

DATE	08/29/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 282771 LOCATION: WATERDOWN
 BUILDER: GREENPARK - RUSSELL GARDENS PH 2 SUB-BUILDER:
 MODEL: ROSEWOOD 6 ELEVATION: 1

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
4	Hangers	HGUS26-2	
7	Hangers	LJS26DS	

TOTAL # ITEMS= 11.00



Delivery Shiplist

DATE	08/29/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 282772 LOCATION: WATERDOWN
 BUILDER: GREENPARK - RUSSELL GARDENS PH 2 SUB-BUILDER:
 MODEL: ROSEWOOD 6 ELEVATION: 2

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY PLY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER TOP BOT	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	1 2 Ply	T1 HIP GIRDER	6.00 0.00	31-11-00	04-01-04	2 X 4 2 X 6	01-03-08 01-03-08	01-02-00 01-02-00	317.54 194.68		
	1	T2 HIP	6.00 0.00	31-11-00	05-01-04	2 X 4 2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	127.46 79.83		
	1	T3 HIP	6.00 0.00	31-11-00	06-01-04	2 X 4 2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	130.07 81.34		
	1	T4 HIP	6.00 0.00	31-11-00	07-01-04	2 X 4 2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	131.31 81.33		
	1	T5 HIP	6.00 0.00	31-11-00	08-01-04	2 X 4 2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	132.23 83.17		
	8	T6 COMMON	6.00 0.00	31-11-00	09-01-12	2 X 4 2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	1041.60 644.00		
	1	T10Z HIP GIRDER	10.00 0.00	31-11-00	04-10-07	2 X 6 2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	191.62 113.34		
	2	T11 HIP	10.00 0.00	31-11-00	06-06-07	2 X 4 2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	271.96 168.34		
	4	T12 HIP	10.00 0.00	31-11-00	08-02-07	2 X 4 2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	612.44 388.00		
	1	T7 HIP GIRDER	6.00 0.00	18-00-00	04-01-04	2 X 4 2 X 6	01-03-08 01-03-08	01-02-00 01-02-00	81.74 52.67		
	1	T8 HIP	6.00 0.00	18-00-00	05-01-04	2 X 4 2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	75.17 48.50		
	3	T20 COMMON	10.00 0.00	18-07-00	09-04-10	2 X 4 2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	253.50 161.49		
	2	T21 COMMON	10.00 0.00	13-00-00	07-00-11	2 X 4 2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	124.22 79.00		
	2	T21S SCISSOR	10.00 5.00	13-00-00	07-00-11	2 X 4 2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	121.94 78.00		
	2	T18 COMMON	7.00 0.00	07-08-00	02-09-11	2 X 4 2 X 4	01-03-08 01-03-08	00-06-14 00-06-14	51.16 35.00		
	1	T18Z COMMON	7.00 0.00	07-08-00	02-09-11	2 X 4 2 X 4	01-03-08 01-03-08	00-06-14 00-06-14	25.58 17.50		
	9	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4 2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	151.11 96.03		
	3	J2 JACK-OPEN	6.00 0.00	05-10-08	02-00-12	2 X 4 2 X 4	01-03-08 -04-01-01	01-02-00 00-03-08	48.48 30.99		



Delivery Shiplist

DATE	08/29/17
SALES REP	Mario

JOB TRACK:43954 LAYOUT ID: 282772 LOCATION: WATERDOWN
 BUILDER: GREENPARK - RUSSELL GARDENS PH 2 SUB-BUILDER:
 MODEL: ROSEWOOD 6 ELEVATION: 2

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	3	J3 JACK-OPEN	6.00 0.00	05-10-08	03-00-12	2 X 4	2 X 4	01-03-08 -02-01-01	01-02-00 00-03-08	56.13 35.01		
	3	J4 JACK-OPEN	6.00 0.00	01-10-08	02-00-12	2 X 4	2 X 4	01-03-08 -00-01-01	01-02-00 00-03-08	25.29 17.01		
	3	J5 JACK-OPEN	6.00 0.00	01-10-08	03-00-12	2 X 4	2 X 4	01-03-08 01-10-15	01-02-00 00-04-11	28.71 18.00		
	2	J6 JACK-OPEN	10.00 0.00	03-10-08	04-10-07	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 04-10-07	27.50 17.34		
	1	J7 JACK-OPEN	10.00 0.00	03-10-08	03-01-09	2 X 4	2 X 4	01-03-08 -02-01-01	01-07-11 00-03-08	13.73 9.33		
	1	J8 JACK-OPEN	10.00 0.00	01-10-08	03-01-09	2 X 4	2 X 4	01-03-08 -00-01-01	01-07-11 00-03-08	9.99 7.00		
	7	J10C JACK-OPEN	6.00 0.00	05-09-00	03-02-11	2 X 4	2 X 4	01-03-08 00-00-00	00-04-03 03-02-11	121.45 77.00		
	3	J12 JACK-OPEN	10.00 0.00	02-03-00	02-09-10	2 X 4	2 X 4	01-03-08 00-00-00	00-11-02 02-09-10	26.01 18.00		
	3	J13C JACK-OPEN	6.00 0.00	04-09-00	02-08-11	2 X 4	2 X 4	01-03-08 00-00-00	00-04-03 02-08-11	44.79 27.00		
	3	J23 JACK-OPEN	5.00 0.00	05-05-00	02-07-02	2 X 4	2 X 4	01-03-08 00-00-00	00-04-01 02-07-02	43.80 27.99		

TOTAL # TRUSS= 74.00

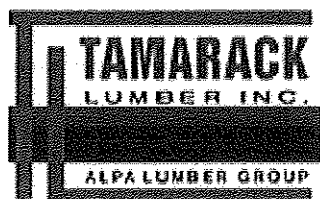
TOTAL BFT OF ALL TRUSSES=

2686.89 BFT. TOTAL WEIGHT OF ALL TRUSSES= 4286.53 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
2	Hangers	LUS24-2	
4	Hangers	LJS26DS	

TOTAL # ITEMS= 8.00



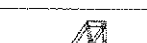
Delivery Shiplist

DATE	08/29/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 282773 LOCATION: WATERDOWN
 BUILDER: GREENPARK - RUSSELL GARDENS PH 2 SUB-BUILDER:
 MODEL: ROSEWOOD 6 ELEVATION: 3

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	T1 HIP GIRDER	6.00	31-11-00	04-01-04	2 X 4	2 X 6	01-03-08	01-02-00	317.54		
	2 Ply		0.00							194.68		
	1	T2 HIP	6.00	31-11-00	05-01-04	2 X 4	2 X 4	01-03-08	01-02-00	127.46		
			0.00					01-03-08	01-02-00	79.83		
	1	T3 HIP	6.00	31-11-00	06-01-04	2 X 4	2 X 4	01-03-08	01-02-00	130.07		
			0.00					01-03-08	01-02-00	81.34		
	1	T4 HIP	6.00	31-11-00	07-01-04	2 X 4	2 X 4	01-03-08	01-02-00	131.31		
			0.00					01-03-08	01-02-00	81.33		
	1	T5 HIP	6.00	31-11-00	08-01-04	2 X 4	2 X 4	01-03-08	01-02-00	132.23		
			0.00					01-03-08	01-02-00	83.17		
	9	T6 COMMON	6.00	31-11-00	09-01-12	2 X 4	2 X 4	01-03-08	01-02-00	1171.80		
			0.00					01-03-08	01-02-00	724.50		
	1	T10Z2 HIP GIRDER	10.00	31-11-00	04-10-07	2 X 6	2 X 6	01-03-08	01-07-11	191.62		
			0.00					01-03-08	01-07-11	113.34		
	2	T11 HIP	10.00	31-11-00	06-06-07	2 X 4	2 X 4	01-03-08	01-07-11	271.96		
			0.00					01-03-08	01-07-11	168.34		
	3	T12 HIP	10.00	31-11-00	08-02-07	2 X 4	2 X 4	01-03-08	01-07-11	459.33		
			0.00					01-03-08	01-07-11	291.00		
	1	T30 HIP GIRDER	10.00	25-01-00	06-10-07	2 X 4	2 X 6	01-03-08	01-07-11	142.43		
			10.00					00-00-00	01-07-11	91.67		
	1	T7 HIP GIRDER	6.00	18-00-00	04-01-04	2 X 4	2 X 6	01-03-08	01-02-00	81.74		
			0.00					01-03-08	01-02-00	52.67		
	1	T8 HIP	6.00	18-00-00	05-01-04	2 X 4	2 X 4	01-03-08	01-02-00	75.17		
			0.00					01-03-08	01-02-00	48.50		
	1	T14Z HIP GIRDER	10.00	13-00-00	04-10-07	2 X 4	2 X 6	01-03-08	01-07-11	68.14		
			0.00					01-03-08	01-07-11	44.00		
	1	T31 HIP	10.00	13-00-00	06-06-07	2 X 4	2 X 4	01-03-08	01-07-11	64.75		
			0.00					01-03-08	01-07-11	41.50		
	1	T21A COMMON	10.00	12-07-00	07-00-11	2 X 4	2 X 4	01-03-08	01-07-11	59.28		
			0.00					00-00-00	01-11-14	38.17		
	3	T32 SCISSORS	10.00	07-08-00	04-10-00	2 X 4	2 X 4	01-03-08	01-07-11	113.52		
			5.00					01-03-08	01-07-11	72.51		
	1	T33 MONOPITCH	10.00	04-04-00	03-07-11	2 X 4	2 X 4	01-03-08	01-07-11	21.75		
			10.00					00-00-00	01-07-11	16.17		
	1	T34 MONOPITCH	10.00	04-04-00	05-03-00	2 X 4	2 X 4	00-00-00	01-07-11	23.00		
			10.00					00-00-00	03-03-00	15.50		



Delivery Shiplist

DATE	08/29/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 282773 LOCATION: WATERDOWN
 BUILDER: GREENPARK - RUSSELL GARDENS PH 2 SUB-BUILDER:
 MODEL: ROSEWOOD 6 ELEVATION: 3

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	9	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	151.11 96.03		
	3	J2 JACK-OPEN	6.00 0.00	05-10-08	02-00-12	2 X 4	2 X 4	01-03-08 -04-01-01	01-02-00 00-03-08	48.48 30.99		
	3	J3 JACK-OPEN	6.00 0.00	05-10-08	03-00-12	2 X 4	2 X 4	01-03-08 -02-01-01	01-02-00 00-03-08	56.13 35.01		
	3	J4 JACK-OPEN	6.00 0.00	01-10-08	02-00-12	2 X 4	2 X 4	01-03-08 -00-01-01	01-02-00 00-03-08	25.29 17.01		
	3	J5 JACK-OPEN	6.00 0.00	01-10-08	03-00-12	2 X 4	2 X 4	01-03-08 01-10-15	01-02-00 00-04-11	27.93 17.49		
	8	J6 JACK-OPEN	10.00 0.00	03-10-08	04-10-07	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 04-10-07	110.00 69.36		
	2	J7 JACK-OPEN	10.00 0.00	03-10-08	03-01-09	2 X 4	2 X 4	01-03-08 -02-01-01	01-07-11 00-03-08	27.46 18.66		
	2	J8 JACK-OPEN	10.00 0.00	01-10-08	03-01-09	2 X 4	2 X 4	01-03-08 -00-01-01	01-07-11 00-03-08	19.98 14.00		
	4	J10C JACK-OPEN	6.00 0.00	05-09-00	03-02-11	2 X 4	2 X 4	01-03-08 00-00-00	00-04-03 03-02-11	69.40 44.00		
	6	J13C JACK-OPEN	6.00 0.00	04-09-00	02-08-11	2 X 4	2 X 4	01-03-08 00-00-00	00-04-03 02-08-11	89.58 54.00		

TOTAL # TRUSS= 75.00

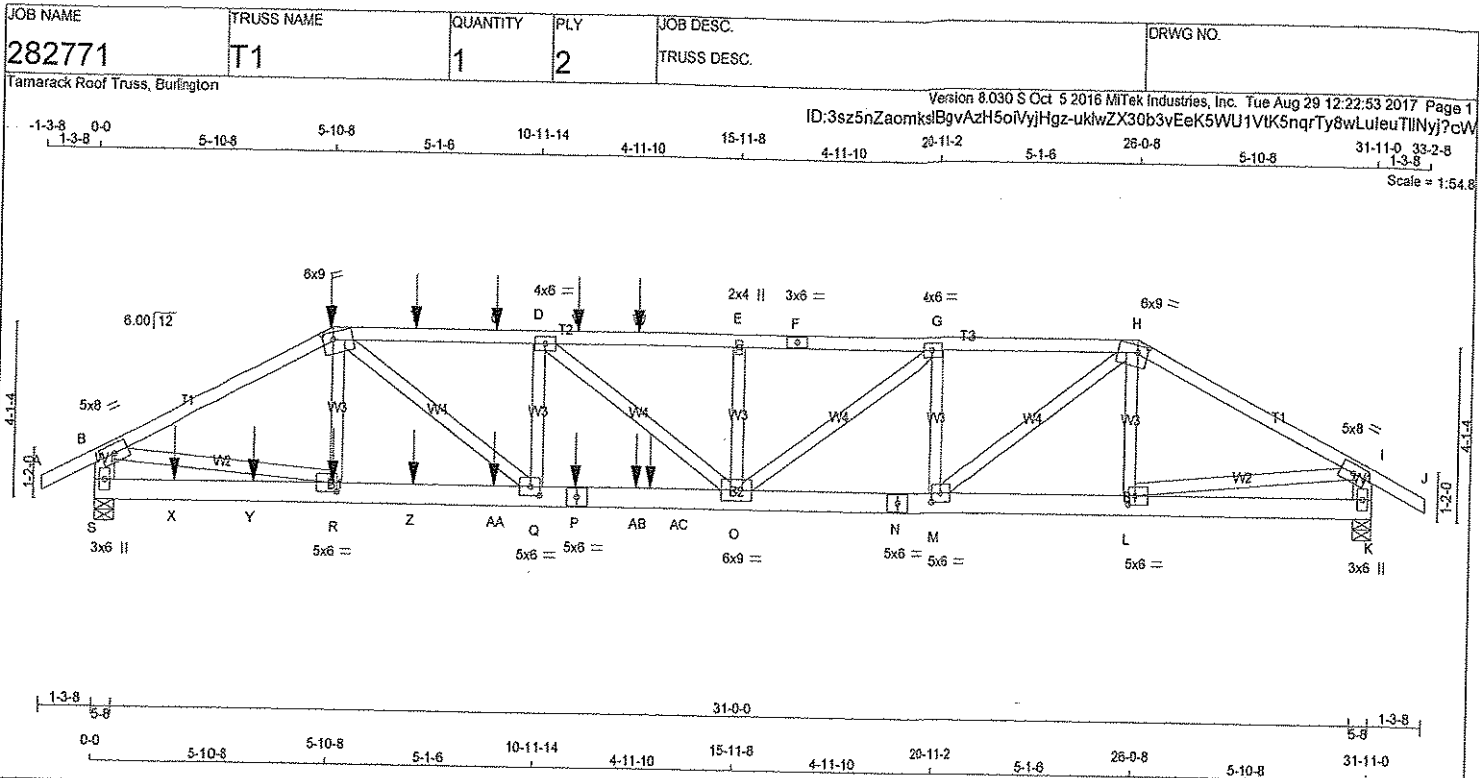
TOTAL BFT OF ALL TRUSSES=

2634.77 BFT. TOTAL WEIGHT OF ALL TRUSSES= 4208.46 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
1	Hangers	HUSC26-2	
2	Hangers	LUS24-2	
5	Hangers	LUS24	
3	Hangers	LJS26DS	

TOTAL # ITEMS= 13.00



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2 SPF
C - F	2x4	DRY	No.2 SPF
F - H	2x4	DRY	No.2 SPF
H - J	2x4	DRY	No.2 SPF
S - B	2x6	DRY	No.2 SPF
K - I	2x6	DRY	No.2 SPF
S - P	2x6	DRY	1650F 1.5E SPF
P - N	2x6	DRY	1650F 1.5E SPF
N - K	2x6	DRY	1650F 1.5E SPF

ALL WEBS 2x4 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C	12	SIDE(61.0)
C-F	12	SIDE(61.0)
F-H	12	TOP
H-J	12	TOP
S-B	12	TOP
K-I	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
S-P	12	SIDE(183.1)
P-N	12	SIDE(0.0)
N-K	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
R-C	6	SIDE(1.2)
2x4	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-I	MT20	5.0	8.0	
C	TTWW-m	MT20	6.0	9.0	2.00 3.00
D	TMVW-I	MT20	4.0	6.0	
E	TMVW-w	MT20	2.0	4.0	
F	TS-I	MT20	3.0	8.0	
G	TMVW-I	MT20	4.0	6.0	
H	TTWW-m	MT20	6.0	9.0	2.00 3.00
I	TMVW-I	MT20	5.0	8.0	
K	BMV1+p	MT20	3.0	6.0	
L	BMVW-I	MT20	5.0	6.0	2.50 2.00
M	BMVW-I	MT20	5.0	6.0	2.50 2.75
N	BS-I	MT20	5.0	6.0	
O	BMVWVW-I	MT20	6.0	9.0	
P	BS-I	MT20	5.0	6.0	
Q	BMVW-I	MT20	5.0	6.0	2.50 2.75
R	BMVW-I	MT20	5.0	6.0	2.50 2.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS					
	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG	
JT	VERT	DOWN	UPLIFT	IN-SX	IN-SX
S	4065	0	4065	0	5-8
K	3175	0	3175	0	5-8

UNFACTORED REACTIONS						
JT	1ST LCASE	MAX/MIN	COMPONENT REACTIONS			
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
S	3304	1887 / 0	716 / 0	0 / 0	0 / 0	702 / 0
K	2572	1483 / 0	549 / 0	0 / 0	0 / 0	541 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.68 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

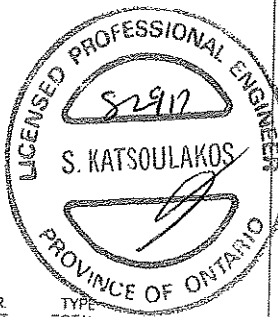
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED (PLF)	MEMB.	FORCE (LBS)	MAX. FACTORED (LBS)	MAX. FACTORED (LC)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 26	-84.3	-84.3 0.06 (1)	10.00	R-C	-840 / 98	0.06 (1)
B-C	-6304 / 0	-84.3	-84.3 0.59 (1)	3.48	C-Q	0 / 4251	0.38 (1)
C-T	-9048 / 0	-84.3	-84.3 0.70 (1)	2.76	Q-D	-1114 / 8	0.10 (1)
T-U	-9048 / 0	-84.3	-84.3 0.70 (1)	2.76	D-O	0 / 554	0.05 (1)
U-D	-9048 / 0	-84.3	-84.3 0.70 (1)	2.76	O-E	-428 / 0	0.04 (1)
D-V	-9499 / 0	-84.3	-84.3 0.73 (1)	2.66	E-G	0 / 2894	0.26 (1)
V-W	-9499 / 0	-84.3	-84.3 0.73 (1)	2.66	M-G	-2204 / 0	0.20 (1)
W-E	-9499 / 0	-84.3	-84.3 0.73 (1)	2.66	H-H	0 / 3523	0.31 (1)
E-F	-9499 / 0	-84.3	-84.3 0.59 (1)	2.82	L-H	-328 / 64	0.03 (1)
F-G	-9499 / 0	-84.3	-84.3 0.59 (1)	2.82	B-R	0 / 5682	0.50 (1)
G-H	-7184 / 0	-84.3	-84.3 0.42 (1)	3.40	L-I	0 / 4384	0.39 (1)
H-I	-4864 / 0	-84.3	-84.3 0.42 (1)	3.99			
I-J	0 / 26	-84.3	-84.3 0.06 (1)	10.00			
S-B	-3913 / 0	0.0	0.0 0.14 (1)	7.16			
K-I	-3101 / 0	0.0	0.0 0.11 (1)	7.81			

FACTORED CONCENTRATED LOADS (LBS)					
JT	LOC.	LC1	MAX.	FACE	DIR.
C	5-10-8	-472	-472	FRONT	VERT
P	11-11-4	-40	-70	FRONT	VERT
R	5-11-4	-40	-70	FRONT	VERT
T	7-11-4	-101	-101	FRONT	VERT
U	9-11-4	-101	-101	FRONT	VERT
V	11-11-4	-101	-101	FRONT	VERT
W	13-5-4	-101	-101	FRONT	VERT



DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.06")
CALCULATED VERT. DEFL.(LL) = L/989 (0.32")
ALLOWABLE DEFL.(TL) = L/360 (1.06")
CALCULATED VERT. DEFL.(TL) = L/756 (0.51")

CSI: TC=0.73 (D-E-1), BC=0.92 (O-Q-1), WB=0.50 (B-R-1), SSI=0.46 (O-Q-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.90 (F) (INPUT = 0.90)
SI METAL= 0.81 (N) (INPUT = 1.00)

DWG NO. TAM 43550-17
STRUCTURAL
COMPONENT ONLY
CONTINUED ON PAGE 2

JOB NAME 282771	TRUSS NAME T1	QUANTITY 1	PLY 2	JOB DESC. TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.030 5 Oct 5 2016 MiTek Industries, Inc. Tue Aug 29 12:22:53 2017 Page 2 ID:3sz5nZaomkslBgVzH5oiVylHgz-ukwZX30b3vEeK5WU1VlK5nqrTy8wLuleuTlINyl?cV	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
S	BMV1+p	MT20	3.0	6.0		

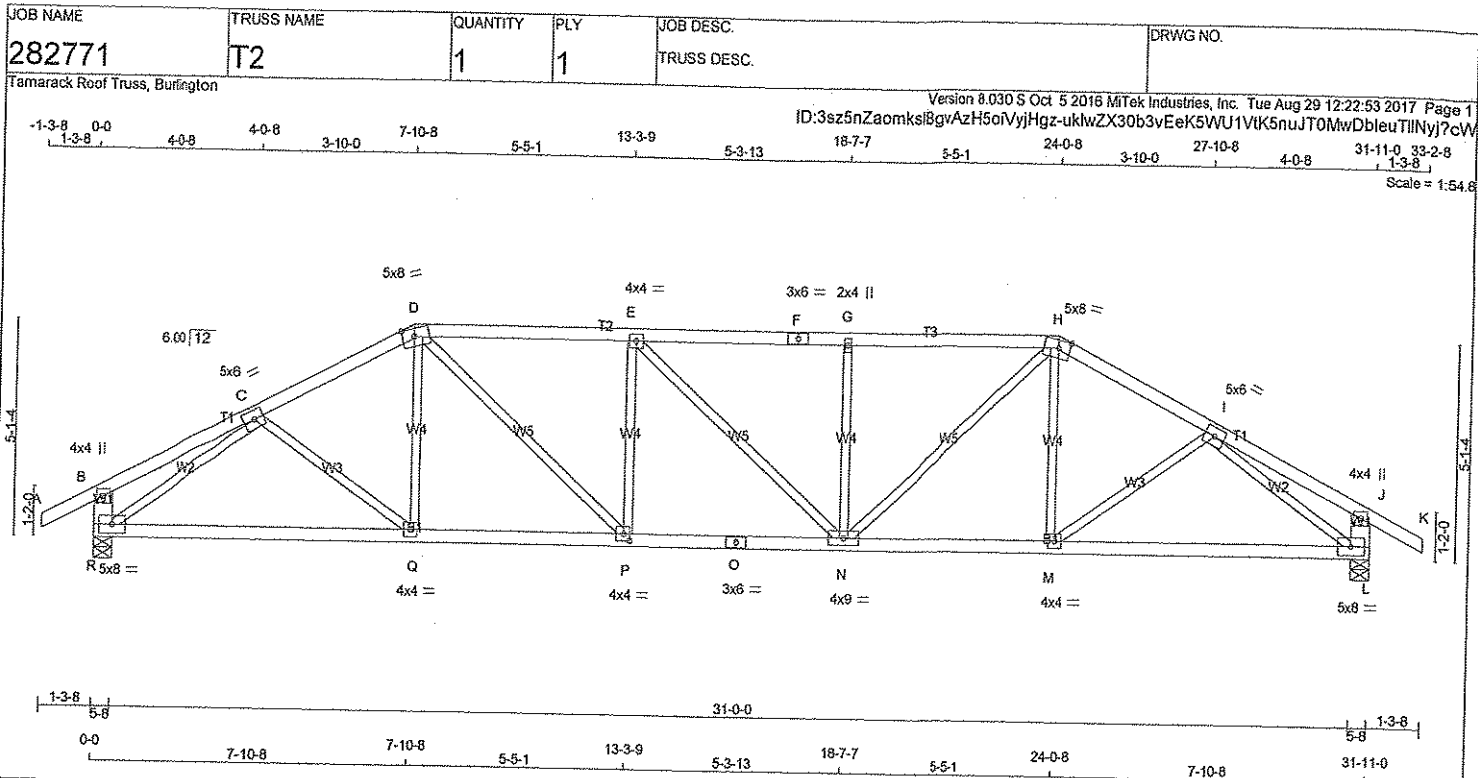
HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 472.3 lbs FACTORED DOWN AT 5-10-8, 101.4 lbs FACTORED DOWN AT 7-11-4, 101.4 lbs FACTORED DOWN AT 9-11-4, AND 101.4 lbs FACTORED DOWN AT 11-11-4, AND 101.4 lbs FACTORED DOWN AT 13-5-4 ON TOP CHORD, AND 84.6 lbs FACTORED DOWN AT 1-11-4, 84.6 lbs FACTORED DOWN AT 3-11-4, 69.9 lbs FACTORED DOWN AT 5-11-4, 69.9 lbs FACTORED DOWN AT 7-11-4, 69.9 lbs FACTORED DOWN AT 9-11-4, 69.9 lbs FACTORED DOWN AT 11-11-4, AND 69.9 lbs FACTORED DOWN AT 13-5-4, AND 2253.6 lbs FACTORED DOWN AT 13-9-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)						
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.
X	1-11-4	-48	-85	---	FRONT	VERT
Y	3-11-4	-48	-85	---	FRONT	VERT
Z	7-11-4	-40	-70	---	FRONT	VERT
AA	9-11-4	-40	-70	---	FRONT	VERT
AB	13-5-4	-40	-70	---	FRONT	VERT
AC	13-9-8	-2254	-2254	---	FRONT	VERT
						TOTAL



DWG NO. TAM 43550-17
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
R - B	2x6	DRY	No.2
L - J	2x6	DRY	No.2
R - O	2x4	DRY	No.2
O - L	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW-m	MT20	5.0	8.0	2.25	3.50
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	8.0	2.25	3.50
I	TMWW-t	MT20	5.0	8.0		
J	TMV+p	MT20	4.0	4.0		
L	BMVW-t	MT20	5.0	8.0		
M	BMVW-t	MT20	4.0	4.0		
N	BMVWW-t	MT20	4.0	9.0		
O	BS-t	MT20	3.0	6.0		
P	BMVW-t	MT20	4.0	4.0	2.00	1.75
Q	BMVW-t	MT20	4.0	4.0		
R	BMVW-t	MT20	5.0	8.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
R	1906 0	1906 0	5-8	5-8
L	1906 0	1906 0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
R	1548	886 / 0	335 / 0	0 / 0	0 / 0	327 / 0	0 / 0	0 / 0
L	1548	886 / 0	335 / 0	0 / 0	0 / 0	327 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.62 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	CS1 (LC)	UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	CS1 (LC)
FR-TO	A-B	0 / 26	-84.3	-84.3	0.11 (1)	10.00	C-Q	0 / 192	0.04 (3)	
	B-C	0 / 14	-84.3	-84.3	0.17 (1)	10.00	Q-D	0 / 295	0.07 (2)	
	C-D	-2523 / 0	-84.3	-84.3	0.27 (1)	4.11	D-P	0 / 932	0.21 (1)	
	D-E	-2943 / 0	-84.3	-84.3	0.51 (1)	3.62	P-E	-490 / 0	0.19 (1)	
	E-F	-2942 / 0	-84.3	-84.3	0.50 (1)	3.62	E-N	-2 / 0	0.00 (1)	
	F-G	-2942 / 0	-84.3	-84.3	0.50 (1)	3.62	N-G	-489 / 0	0.19 (1)	
	G-H	-2941 / 0	-84.3	-84.3	0.50 (1)	3.63	H-N	0 / 930	0.21 (1)	
	H-I	-2523 / 0	-84.3	-84.3	0.27 (1)	4.11	M-H	0 / 295	0.07 (2)	
	I-J	0 / 14	-84.3	-84.3	0.17 (1)	10.00	M-I	0 / 192	0.04 (3)	
	J-K	0 / 26	-84.3	-84.3	0.11 (1)	10.00	R-C	-2684 / 0	0.97 (1)	
	R-B	-249 / 0	0.0	0.0	0.02 (1)	7.81	I-L	-2684 / 0	0.97 (1)	
	L-J	-249 / 0	0.0	0.0	0.02 (1)	7.81				
	R-Q	0 / 2181	-28.0	-28.0	0.62 (2)	10.00				
	Q-P	0 / 2245	-28.0	-28.0	0.65 (2)	10.00				
	P-O	0 / 2943	-28.0	-28.0	0.56 (1)	10.00				
	O-N	0 / 2943	-28.0	-28.0	0.56 (1)	10.00				
	N-M	0 / 2246	-28.0	-28.0	0.65 (2)	10.00				
	M-L	0 / 2181	-28.0	-28.0	0.62 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 3.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014

- CSA 086-09

- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.06")

CALCULATED VERT. DEFL.(LL) = L/999 (0.20")

ALLOWABLE DEFL.(TL) = L/360 (1.06")

CALCULATED VERT. DEFL.(TL) = L/999 (0.33")

CSI: TC=0.51 (D-E:1), BC=0.65 (M-N:2), WB=0.97 (I-L:1), SSI=0.21 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

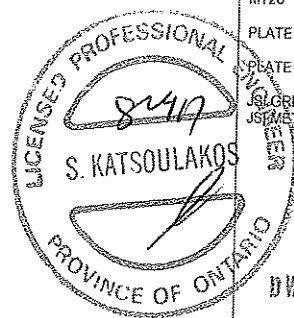
PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

SLIP= 0.87 (I) (INPUT = 0.90)

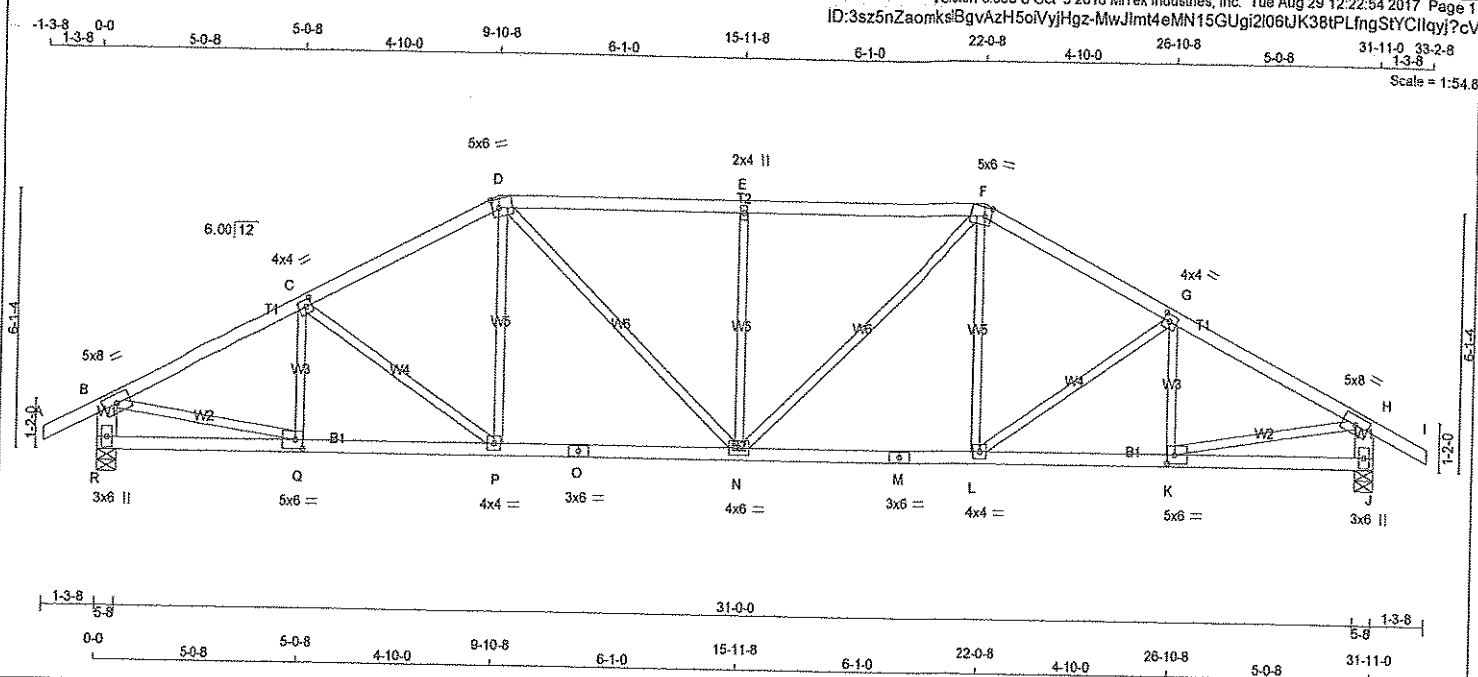
JSI/METAL= 0.87 (O) (INPUT = 1.00)



DRWG NO. TAM 43551-17

STRUCTURAL

COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
R - B	2x6	DRY	No.2
J - H	2x6	DRY	No.2
R - O	2x4	DRY	No.2
O - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-I	MT20	5.0	8.0		
C	TMVW-I	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	6.0	2.50	2.00
E	TMVW-w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.50	2.00
G	TMVW-I	MT20	4.0	4.0	2.00	1.75
H	TMVW-I	MT20	5.0	8.0		
J	BMV1+p	MT20	3.0	6.0		
K	BMVW-I	MT20	5.0	6.0	2.50	2.25
L	BMVW-I	MT20	4.0	4.0		
M	BS-I	MT20	3.0	6.0		
N	BMVWVW-I	MT20	4.0	6.0		
O	BS-I	MT20	3.0	6.0		
P	BMVWVW-I	MT20	4.0	4.0		
Q	BMVWVW-I	MT20	5.0	6.0	2.50	2.25
R	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	UPLIFT	IN-SX
R	1906	0	0	5-8
J	1906	0	0	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
R	1548	886 / 0	335 / 0	0 / 0	0 / 0	327 / 0	0 / 0	0 / 0
J	1548	886 / 0	335 / 0	0 / 0	0 / 0	327 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.85 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 26	-84.3 -84.3	0.11 (1)	10.00	Q-C	-255 / 66	0.06 (1)
B-C	-2585 / 0	-84.3 -84.3	0.35 (1)	4.01	C-P	-264 / 0	0.16 (1)
C-D	-2385 / 0	-84.3 -84.3	0.33 (1)	4.17	P-D	0 / 364	0.08 (2)
D-E	-2536 / 0	-84.3 -84.3	0.50 (1)	3.85	D-N	0 / 577	0.13 (1)
E-F	-2536 / 0	-84.3 -84.3	0.50 (1)	3.85	N-E	-628 / 0	0.37 (1)
F-G	-2385 / 0	-84.3 -84.3	0.33 (1)	4.17	N-F	0 / 577	0.13 (1)
G-H	-2585 / 0	-84.3 -84.3	0.35 (1)	4.01	L-F	0 / 364	0.08 (2)
H-I	0 / 26	-84.3 -84.3	0.11 (1)	10.00	L-G	-264 / 0	0.16 (1)
R-B	-1843 / 0	0.0 0.0	0.12 (1)	7.38	K-G	-255 / 66	0.06 (1)
J-H	-1843 / 0	0.0 0.0	0.12 (1)	7.38	B-Q	0 / 2364	0.53 (1)
R-Q	0 / 0	-28.0 -28.0	0.15 (2)	10.00	K-H	0 / 2364	0.53 (1)
Q-P	0 / 2331	-28.0 -28.0	0.47 (1)	10.00			
P-O	0 / 2119	-28.0 -28.0	0.46 (2)	10.00			
O-N	0 / 2119	-28.0 -28.0	0.46 (2)	10.00			
N-M	0 / 2119	-28.0 -28.0	0.46 (2)	10.00			
M-L	0 / 2119	-28.0 -28.0	0.46 (2)	10.00			
L-K	0 / 2331	-28.0 -28.0	0.47 (1)	10.00			
K-J	0 / 0	-28.0 -28.0	0.15 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 25.6	PSF
	DL	= 3.0	PSF
BOT CH.	LL	= 10.5	PSF
	DL	= 7.0	PSF
TOTAL LOAD = 46.1 PSF			

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.08")

CALCULATED VERT. DEFL.(LL) = L/999 (0.16")

ALLOWABLE DEFL.(TL)= L/360 (1.06")

CALCULATED VERT. DEFL.(TL) = L/999 (0.25")

CSI: TC=0.50 (E-F:1), BC=0.47 (K-L:1), WB=0.53 (H-K:1), SSI=0.25 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

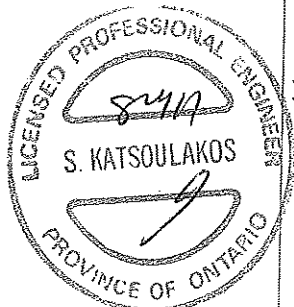
PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX MIN
MT20	618	354	1687 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.87 (D) (INPUT = 0.90)

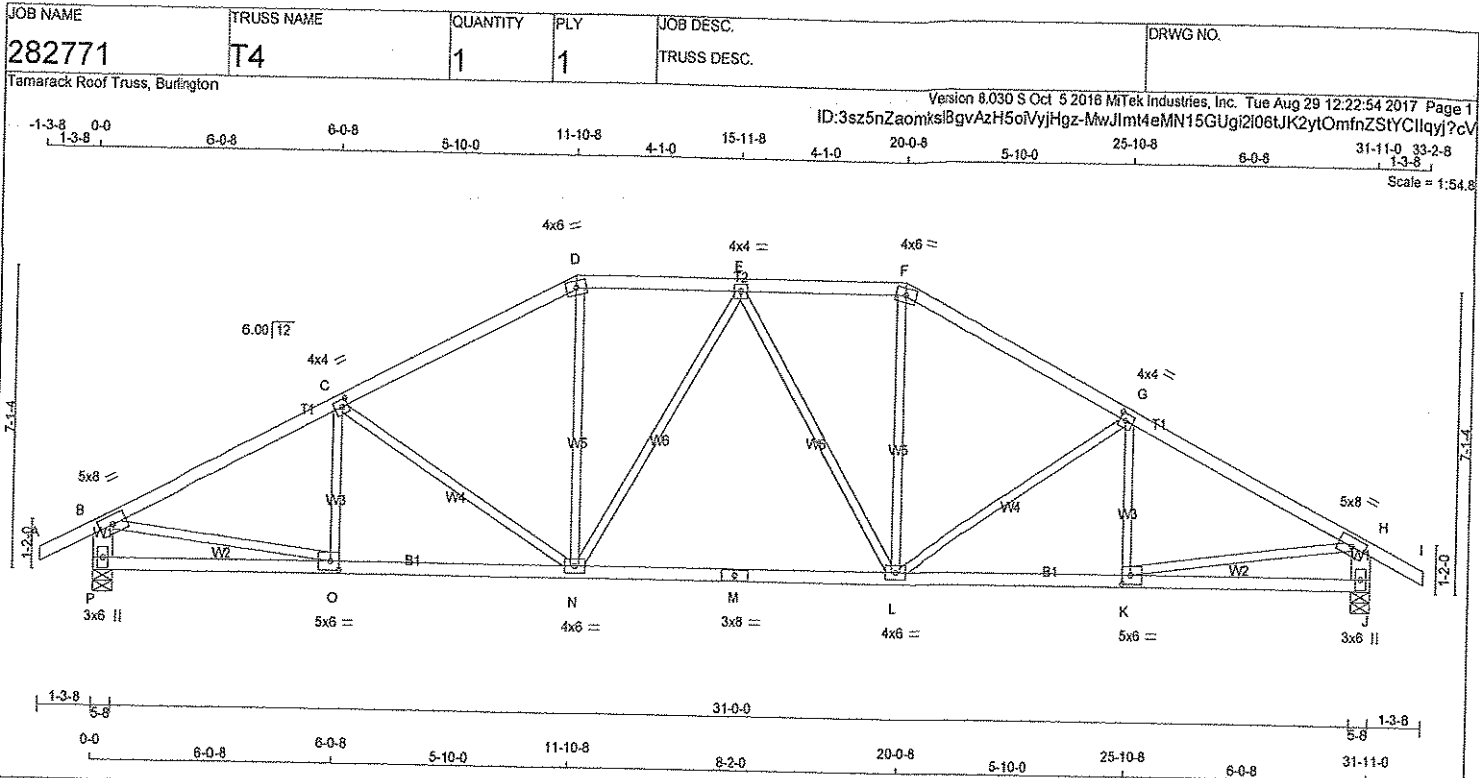
JSI METAL= 0.61 (O) (INPUT = 1.00)



DRWG NO. TAM43552-17

STRUCTURAL

COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
P - B	2x6	DRY	No.2
J - H	2x6	DRY	No.2
P - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTW-m	MT20	4.0	6.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	6.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
H	TMVW-t	MT20	5.0	8.0		
J	BMV1+p	MT20	3.0	6.0		
K	BMWW-t	MT20	5.0	6.0	2.50	2.50
L	BMWW-t	MT20	4.0	6.0		
M	BS-t	MT20	3.0	8.0		
N	BMWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	5.0	6.0	2.50	2.50
P	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	5-8	5-8
P	HORZ	UP/LIFT	5-8	5-8
J	HORZ	UP/LIFT	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX	MIN	COMPONENT REACTIONS
P	1548	886 / 0	335 / 0	0 / 0
J	1548	886 / 0	335 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.81 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO					FR-TO		
A-B	0 / 26	-84.3	-84.3 0.11 (1)	10.00	O-C	-167 / 140	0.04 (1)
B-C	-2623 / 0	-84.3	-84.3 0.52 (1)	3.81	C-N	-469 / 0	0.45 (1)
C-D	-2237 / 0	-84.3	-84.3 0.48 (1)	4.13	N-D	0 / 687	0.15 (1)
D-E	-1988 / 0	-84.3	-84.3 0.21 (1)	4.61	N-E	-239 / 0	0.31 (1)
E-F	-1988 / 0	-84.3	-84.3 0.21 (1)	4.13	E-L	-239 / 0	0.31 (1)
F-G	-2237 / 0	-84.3	-84.3 0.48 (1)	4.13	L-F	0 / 687	0.15 (1)
G-H	-2623 / 0	-84.3	-84.3 0.52 (1)	3.81	L-G	-469 / 0	0.45 (1)
H-I	0 / 26	-84.3	-84.3 0.11 (1)	10.00	K-G	-167 / 140	0.04 (1)
P-B	-1832 / 0	0.0	0.0 0.12 (1)	7.39	B-O	0 / 2392	0.54 (1)
J-H	-1832 / 0	0.0	0.0 0.12 (1)	7.39	K-H	0 / 2392	0.54 (1)
P-O	0 / 0	-28.0	-28.0 0.21 (2)	10.00			
O-N	0 / 2369	-28.0	-28.0 0.57 (2)	10.00			
N-M	0 / 2107	-28.0	-28.0 0.54 (2)	10.00			
M-L	0 / 2107	-28.0	-28.0 0.54 (2)	10.00			
L-K	0 / 2369	-28.0	-28.0 0.57 (2)	10.00			
K-J	0 / 0	-28.0	-28.0 0.21 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.06")
CALCULATED VERT. DEFL.(LL) = L/999 (0.19")
ALLOWABLE DEFL.(TL) = L/360 (1.06")
CALCULATED VERT. DEFL.(TL) = L/999 (0.32")

CSI: TC=0.52 (G-H-1), BC=0.57 (K-L-2), WB=0.54 (H-K-1), SS=0.22 (G-H-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

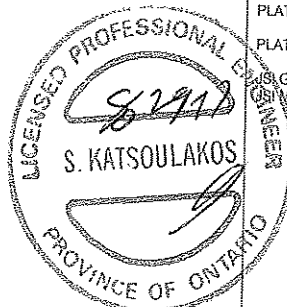
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 822 2284 1656

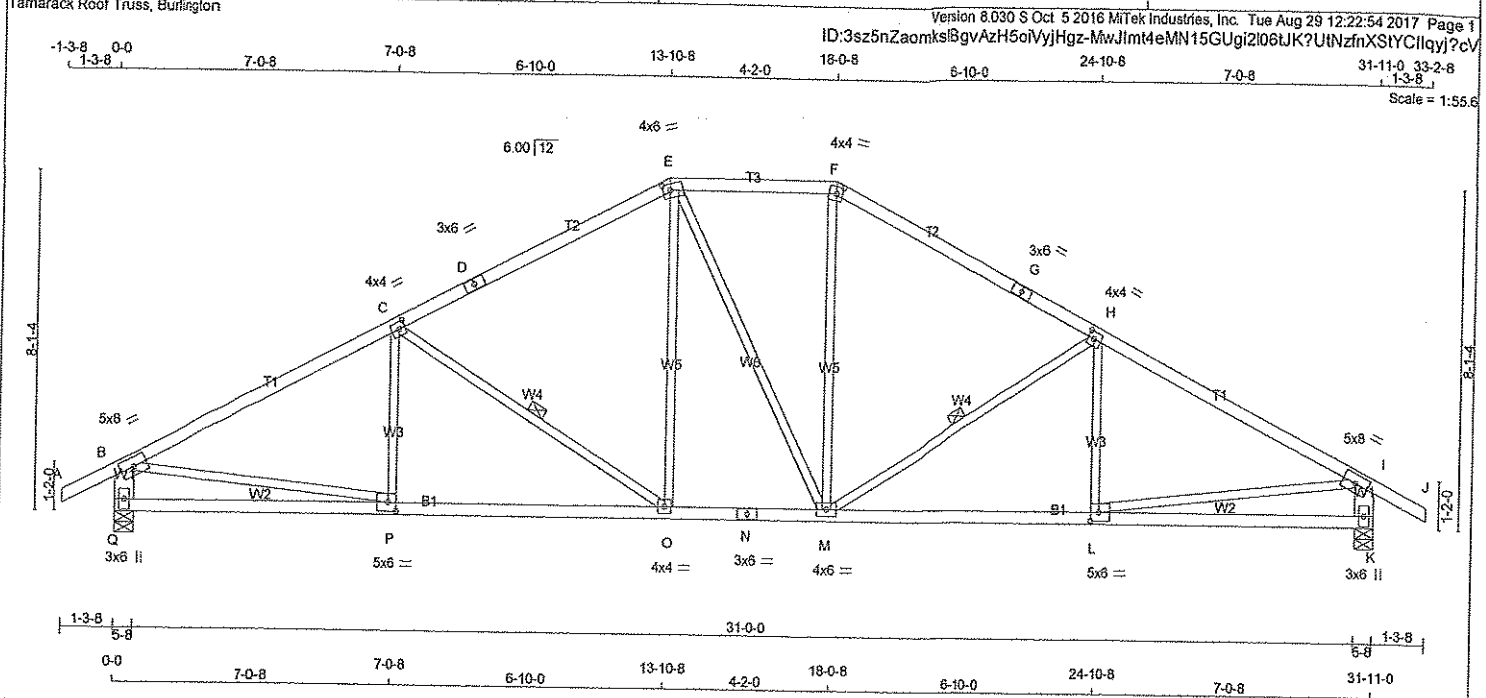
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

GRIP=0.90 (K) (INPUT = 0.90)
METAL=0.77 (M) (INPUT = 1.00)



DWG NO. TAM 43553-17
STRUCTURAL
COMPONENT ONLY



LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.	BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REGRD	SPECIFIED LOADS:		
CHORDS					GROSS REACTION	GROSS REACTION	BRG	BRG	TOP CH. LL =	25.6	PSF
A - D	2x4	DRY	No.2	JT	VERT	HORZ	DOWN	HORZ	BOT CH. LL =	10.5	PSF
D - E	2x4	DRY	No.2	Q	1906	0	1906	0	DL =	7.0	PSF
E - F	2x4	DRY	No.2	K	1906	0	1906	0	TOTAL LOAD =	46.1	PSF
F - G	2x4	DRY	No.2								
G - J	2x4	DRY	No.2								
Q - B	2x6	DRY	No.2								
K - I	2x6	DRY	No.2								
Q - N	2x4	DRY	No.2								
N - K	2x4	DRY	No.2								
ALL WEBS	2x3	DRY	No.2								
EXCEPT											
DRY: SEASONED LUMBER.				UNFACTORED REACTIONS				SPACING = 24.0 IN. C/C			
					1ST LCASE	MAX/MIN. COMPONENT REACTIONS			LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12		
				JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
				Q	1548	886 / 0	335 / 0	0 / 0	0 / 0	327 / 0	0 / 0
				K	1548	886 / 0	335 / 0	0 / 0	0 / 0	327 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.53 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-O, H-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

PLATES (Table is in inches)				CHORDS				WEBS			
JT	TYPE	PLATES	W	LEN	Y	X	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	UNBRACED LENGTH
B	TMVW-t	MT20	5.0	8.0			A-B	0 / 26	-84.3	-84.3	0.11 (1)
C	TMVW-t	MT20	4.0	4.0	2.00	1.75	B-C	-2636 / 0	-84.3	-84.3	0.74 (1)
D	TS-t	MT20	3.0	6.0			C-D	-2048 / 0	-84.3	-84.3	0.63 (1)
E	TTWW-m	MT20	4.0	6.0	1.75	2.25	D-E	-2048 / 0	-84.3	-84.3	0.63 (1)
F	TTWW-m	MT20	4.0	4.0	2.00	1.75	E-F	-1810 / 0	-84.3	-84.3	0.23 (1)
G	TS-t	MT20	3.0	6.0			F-G	-2049 / 0	-84.3	-84.3	0.63 (1)
H	TMVW-t	MT20	4.0	4.0	2.00	1.75	G-H	-2049 / 0	-84.3	-84.3	0.63 (1)
I	TMVW-t	MT20	5.0	8.0			H-I	-2636 / 0	-84.3	-84.3	0.74 (1)
K	BMV1+p	MT20	3.0	6.0			I-J	0 / 26	-84.3	-84.3	0.11 (1)
L	BMVW-t	MT20	5.0	6.0	2.50	2.50	Q-B	-1825 / 0	0.0	0.0	0.12 (1)
M	BMVW-t	MT20	4.0	6.0			K-I	-1825 / 0	0.0	0.0	0.12 (1)
N	BS-t	MT20	3.0	6.0			Q-P	0 / 0	-28.0	-28.0	0.35 (3)
O	BMVW-t	MT20	4.0	4.0			P-O	0 / 2388	-28.0	-28.0	0.62 (2)
P	BMVW-t	MT20	5.0	6.0	2.50	2.50	O-N	0 / 1809	-28.0	-28.0	0.39 (1)
Q	BMV1+p	MT20	3.0	6.0			N-M	0 / 1809	-28.0	-28.0	0.39 (1)
							M-L	0 / 2385	-28.0	-28.0	0.62 (2)
							L-K	0 / 0	-28.0	-28.0	0.35 (3)



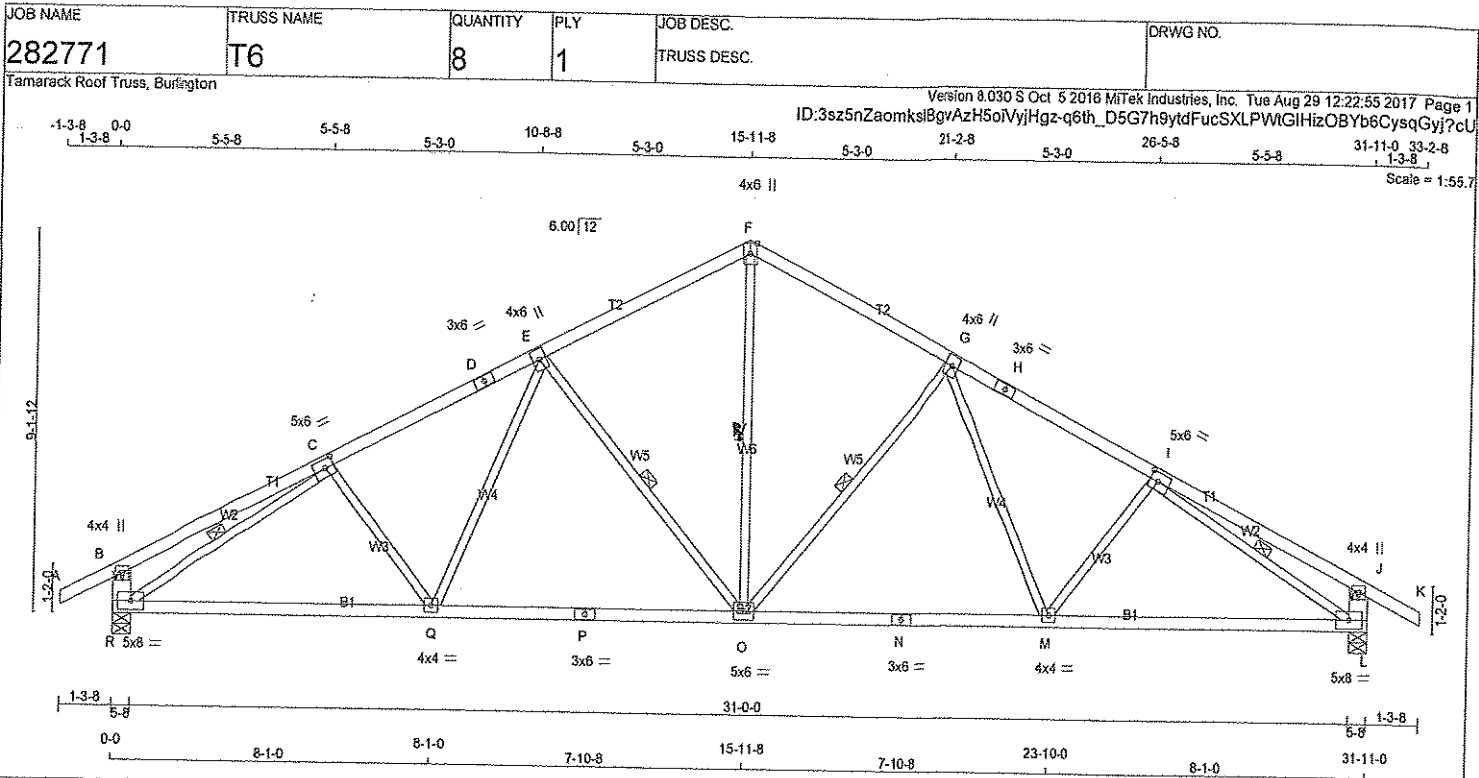
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1867 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg

GRIP = 0.88 (P) (INPUT = 0.90)
METAL = 0.60 (P) (INPUT = 1.00)

DRW NO. TAM 43554-17
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
R - B	2x6	DRY	No.2
L - J	2x6	DRY	No.2
R - P	2x4	DRY	No.2
P - N	2x4	DRY	No.2
N - L	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.50
D	TS-t	MT20	3.0	6.0		
E	TMWW+t	MT20	4.0	6.0		
F	TTW+p	MT20	4.0	6.0	Edge	
G	TMWW+t	MT20	4.0	6.0		
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	5.0	6.0	2.50	2.50
J	TMV+p	MT20	4.0	4.0		
L	BMVW-t	MT20	5.0	6.0		
M	BMVW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	6.0		
O	BMWWW-t	MT20	5.0	6.0		
P	BS-t	MT20	3.0	6.0		
Q	BMWWW-t	MT20	4.0	4.0		
R	BMVW-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
R	1906	0	1906	0	5-8	5-8
L	1906	0	1906	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
R	1548	886 / 0	335 / 0	0 / 0	0 / 0	327 / 0	0 / 0
L	1548	886 / 0	335 / 0	0 / 0	0 / 0	327 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.09 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-O, E-O, C-R, I-L

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO		FROM TO	FR-TO	
A-B	0 / 26	-84.3 -84.3 0.11 (1)	O-F	0 / 1275
B-C	0 / 18	-84.3 -84.3 0.29 (1)	O-G	-700 / 0
C-D	-2485 / 0	-84.3 -84.3 0.35 (1)	G-M	0 / 426
D-E	-2485 / 0	-84.3 -84.3 0.35 (1)	M-I	-111 / 124
E-F	-1843 / 0	-84.3 -84.3 0.32 (1)	E-O	-700 / 0
F-G	-1843 / 0	-84.3 -84.3 0.32 (1)	Q-E	0 / 426
G-H	-2485 / 0	-84.3 -84.3 0.35 (1)	C-Q	-111 / 124
H-I	-2485 / 0	-84.3 -84.3 0.35 (1)	R-C	-2746 / 0
I-J	0 / 18	-84.3 -84.3 0.29 (1)	I-L	-2746 / 0
J-K	0 / 26	-84.3 -84.3 0.11 (1)		
R-B	-300 / 0	0.0 0.0 0.02 (1)		
L-J	-300 / 0	0.0 0.0 0.02 (1)		
R-Q	0 / 2292	-28.0 -28.0 0.70 (2)		
Q-P	0 / 2083	-28.0 -28.0 0.68 (2)		
P-O	0 / 2083	-28.0 -28.0 0.68 (2)		
O-N	0 / 2083	-28.0 -28.0 0.68 (2)		
N-M	0 / 2083	-28.0 -28.0 0.68 (2)		
M-L	0 / 2292	-28.0 -28.0 0.70 (2)		

TOTAL WEIGHT = 8 X 130 = 1042 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.06")
CALCULATED VERT. DEFL. (LL) = L/999 (0.20")
ALLOWABLE DEFL. (TL) = L/360 (1.06")
CALCULATED VERT. DEFL. (TL) = L/999 (0.33")

CSI: TC=0.35 (C-E:1), BC=0.70 (L-M:2), WB=0.75 (I-L:1), SSI=0.19 (L-M:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

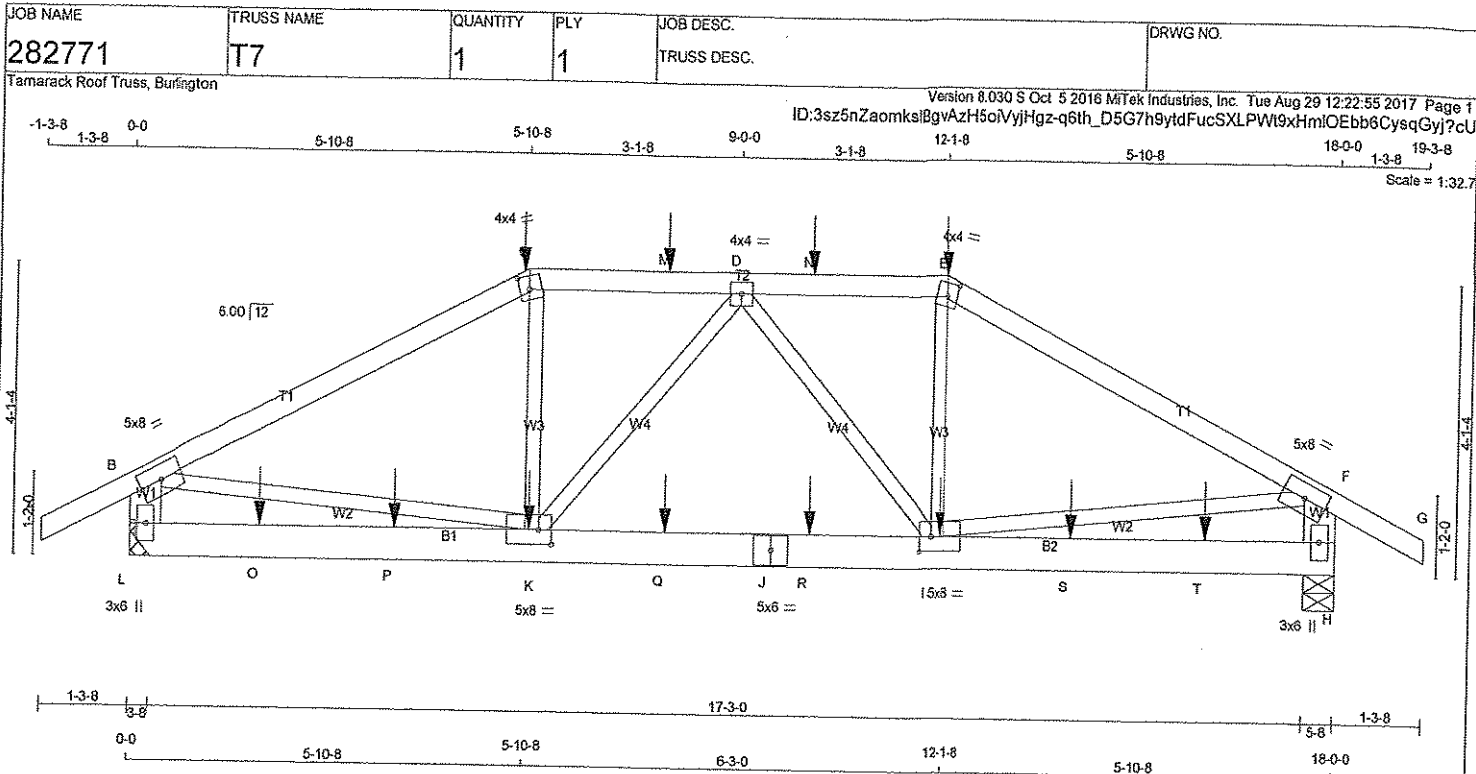
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (I) (INPUT = 0.90)
JSI METAL= 0.68 (C) (INPUT = 1.00)



DRWG NO. TAM 43555-17
STRUCTURAL
COMPONENT ONLY



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
L - B	2x6	DRY	No.2
H - F	2x6	DRY	No.2
L - J	2x6	DRY	No.2
J - H	2x6	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-1	MT20	5.0	8.0	
C	TTW-m	MT20	4.0	4.0	2.25 1.75
E	TMVW-1	MT20	4.0	4.0	
D	TTW-m	MT20	4.0	4.0	2.25 1.75
F	TMVW-1	MT20	5.0	8.0	
H	BMV1+p	MT20	3.0	6.0	
I	BMVWW-1	MT20	5.0	8.0	2.50 2.25
J	BS-1	MT20	5.0	6.0	
K	BMVWW-1	MT20	5.0	8.0	2.50 2.25
L	BMV1+p	MT20	3.0	6.0	

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 472.3 lbs FACTORED DOWN AT 5-10-8, 101.4 lbs FACTORED DOWN AT 7-11-4, AND 101.4 lbs FACTORED DOWN AT 10-0-12, AND 472.3 lbs FACTORED DOWN AT 12-1-8 ON TOP CHORD, AND 84.6 lbs FACTORED DOWN AT 3-11-4, 1-11-4, 84.6 lbs FACTORED DOWN AT 3-11-4, 69.9 lbs FACTORED DOWN AT 5-11-4, 69.9 lbs FACTORED DOWN AT 7-11-4, 69.9 lbs FACTORED DOWN AT 10-0-12, 69.9 lbs FACTORED DOWN AT 12-0-12, AND 84.6 lbs FACTORED DOWN AT 14-0-12, AND 84.6 lbs FACTORED DOWN AT 16-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BUILDING DESIGNER									
BEARINGS									
		FACTORED		MAXIMUM FACTORED			INPUT	REQRD	
		GROSS REACTION		GROSS REACTION			BRG	BRG	
JT		VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
H		1875	0	1875	0	0	5-8	5-8	
L		1875	0	1875	0	0	HANGER BY OTHERS		
							MIN. SEAT SIZE: 3-8		

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
H	1518	878 / 0	321 / 0	0 / 0	0 / 0	320 / 0	0 / 0
L	1518	878 / 0	321 / 0	0 / 0	0 / 0	320 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.43 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED			
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO	FR-TO			
A-B	0 / 26	-84.3	-84.3	0.12 (1)	10.00	K-C	0 / 446
B-C	-2478 / 0	-84.3	-84.3	0.82 (1)	3.43	K-D	-304 / 0
C-M	-2228 / 0	-84.3	-84.3	0.29 (1)	4.24	D-I	-304 / 0
M-D	-2228 / 0	-84.3	-84.3	0.29 (1)	4.24	I-E	0 / 445
D-N	-2228 / 0	-84.3	-84.3	0.29 (1)	4.24	B-K	0 / 2234
N-E	-2228 / 0	-84.3	-84.3	0.29 (1)	4.24	I-F	0 / 2234
E-F	-2478 / 0	-84.3	-84.3	0.82 (1)	3.43		0.55 (1)
F-G	0 / 26	-84.3	-84.3	0.12 (1)	10.00		
L-B	-1758 / 0	0.0	0.0	0.12 (1)	7.46		
H-F	-1758 / 0	0.0	0.0	0.12 (1)	7.46		
L-O	0 / 0	-28.0	-28.0	0.21 (2)	10.00		
O-P	0 / 0	-28.0	-28.0	0.21 (2)	10.00		
P-K	0 / 0	-28.0	-28.0	0.21 (2)	10.00		
K-Q	0 / 2417	-28.0	-28.0	0.40 (2)	10.00		
Q-J	0 / 2417	-28.0	-28.0	0.40 (2)	10.00		
J-R	0 / 2417	-28.0	-28.0	0.40 (2)	10.00		
R-I	0 / 2417	-28.0	-28.0	0.40 (2)	10.00		
I-S	0 / 0	-28.0	-28.0	0.21 (2)	10.00		
S-T	0 / 0	-28.0	-28.0	0.21 (2)	10.00		
T-H	0 / 0	-28.0	-28.0	0.21 (2)	10.00		

LICENSED PROFESSIONAL

82417

S. KATSOULA

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.
C	5-10-8	-472	-472	-	FRONT	VERT
E	12-1-8	-472	-472	-	FRONT	VERT
I	12-0-12	-40	-70	-	FRONT	VERT
K	5-11-4	-40	-70	-	FRONT	VERT
M	7-11-4	-101	-101	-	FRONT	VERT
N	10-0-12	-101	-101	-	FRONT	VERT
O	1-11-4	-48	-85	-	FRONT	VERT
P	3-11-4	-48	-85	-	FRONT	VERT
Q	7-11-4	-40	-70	-	FRONT	VERT
R	10-0-12	-40	-70	-	FRONT	VERT
S	14-0-12	-48	-85	-	FRONT	VERT
T	16-0-12	-48	-85	-	FRONT	VERT

TOTAL WEIGHT = 82 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.60")
CALCULATED VERT. DEFL.(LL)= L/999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.60")
CALCULATED VERT. DEFL.(TL)= L/999 (0.13")

CSI: TC=0.82 (B-C-1), BC=0.40 (I-K-2), WB=0.55 (B-K-1), SS=0.21 (C-D-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

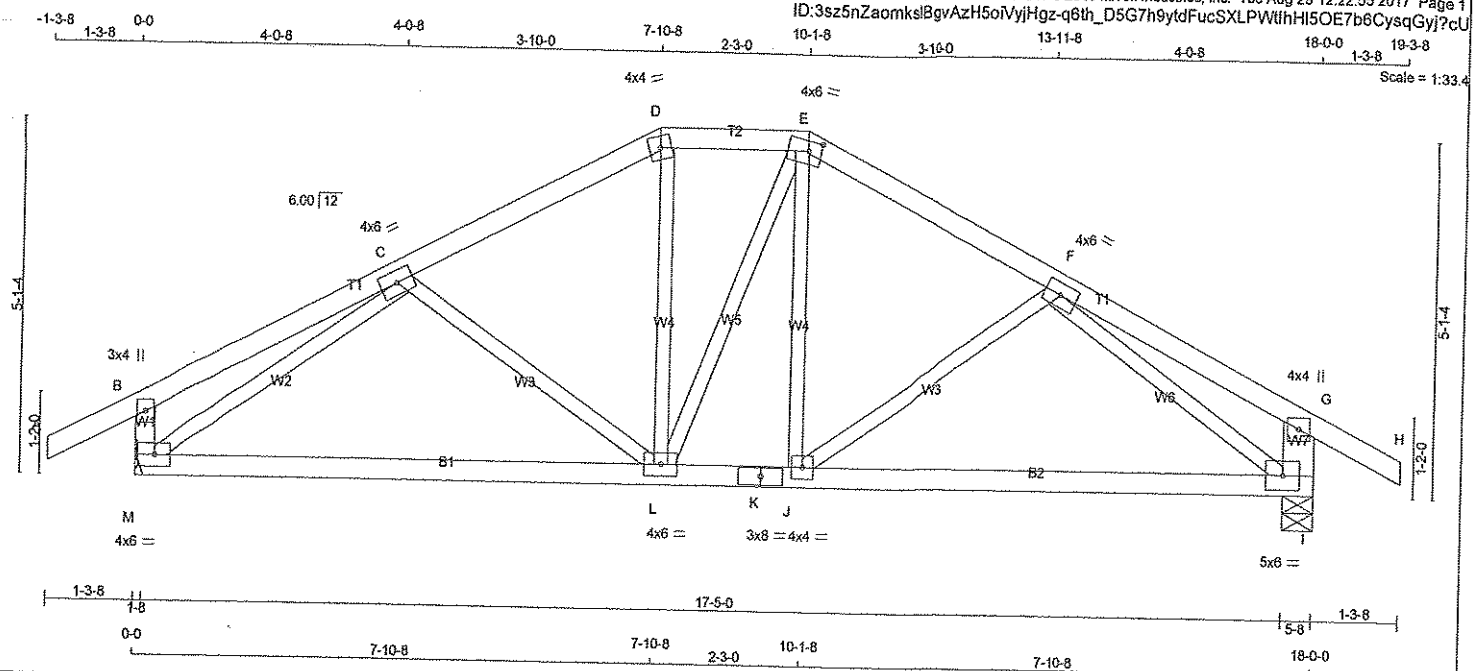
NAIL VALUES
2 PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.49 (F) (INPUT = 1.00)

DWG NO. TAM 43556-17
STRUCTURAL
COMPONENT ONLY



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR
A - D	2x4	DRY No.2	SPF
D - E	2x4	DRY No.2	SPF
E - H	2x4	DRY No.2	SPF
M - B	2x4	DRY No.2	SPF
I - G	2x6	DRY No.2	SPF
M - K	2x4	DRY No.2	SPF
K - I	2x4	DRY No.2	SPF

ALL WEBS 2x3 DRY
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TBM+p	MT20	3.0	4.0	
C	TMMWV-t	MT20	4.0	6.0	
D	TTW-m	MT20	4.0	4.0	
E	TTWW-m	MT20	4.0	6.0	1.75 2.25
F	TMMWV-t	MT20	4.0	6.0	
G	TBM+p	MT20	4.0	4.0	
I	TMMWV-t	MT20	5.0	6.0	
J	TMMWV-t	MT20	4.0	4.0	
K	BS-4	MT20	3.0	8.0	
L	TMMWV-t	MT20	4.0	6.0	
M	TMMWV-t	MT20	4.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED			MAXIMUM FACTORED			INPUT	REQ'D
JT	GROSS REACTION			GROSS REACTION			BRG	BRG
M	VERT	HORZ		DOWN	HORZ	UPLIFT	IN-SX	IN-SX
	1125	0		1125	0	0	HANGER BY OTHERS	
I	1125	0		1125	0	0	MIN. SEAT SIZE: 1-8	
							5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
M	907	530 / 0	189 / 0	0 / 0	0 / 0	188 / 0	0 / 0
I	907	530 / 0	189 / 0	0 / 0	0 / 0	188 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 28	-84.3	-84.3 0.11 (1)	10.00	C-L	-171 / 69	0.07 (1)
B-C	0 / 16	-84.3	-84.3 0.20 (1)	10.00	L-D	0 / 280	0.06 (2)
C-D	-1090 / 0	-84.3	-84.3 0.16 (1)	5.88	L-E	0 / 2	0.00 (2)
D-E	-985 / 0	-84.3	-84.3 0.06 (1)	6.25	J-E	0 / 277	0.06 (2)
E-F	-1089 / 0	-84.3	-84.3 0.16 (1)	5.88	J-F	-171 / 69	0.07 (1)
F-G	0 / 16	-84.3	-84.3 0.20 (1)	10.00	M-C	-1352 / 0	0.52 (1)
G-H	0 / 26	-84.3	-84.3 0.11 (1)	10.00	F-I	-1351 / 0	0.49 (1)
M-B	-245 / 0	0.0	0.0 0.02 (1)	7.81			
I-G	-245 / 0	0.0	0.0 0.02 (1)	7.81			
M-L	0 / 1099	-28.0	-28.0 0.50 (2)	10.00			
L-K	0 / 964	-28.0	-28.0 0.50 (2)	10.00			
K-J	0 / 964	-28.0	-28.0 0.50 (2)	10.00			
J-I	0 / 1098	-28.0	-28.0 0.50 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= $L/360$ (0.60")
 CALCULATED VERT. DEFL.(LL) = $L/999$ (0.16")
 ALLOWABLE DEFL.(TL)= $L/360$ (0.60")
 CALCULATED VERT. DEFL.(TL) = $L/789$ (0.27")

CSI: TC=0.20 (B-C:1), BC=0.50 (J-L:2), WB=0.52 (C-M:1), SSI=0.18 (L-M:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE KEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

	GRIP(DRY)		SHEAR		SECTION	
	(PSI)	(PLI)	(PSI)	(PLI)	(PSI)	(PLI)
MAX	618	1667	822	2284	1656	
MIN	354	822	1656	1656	1656	

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

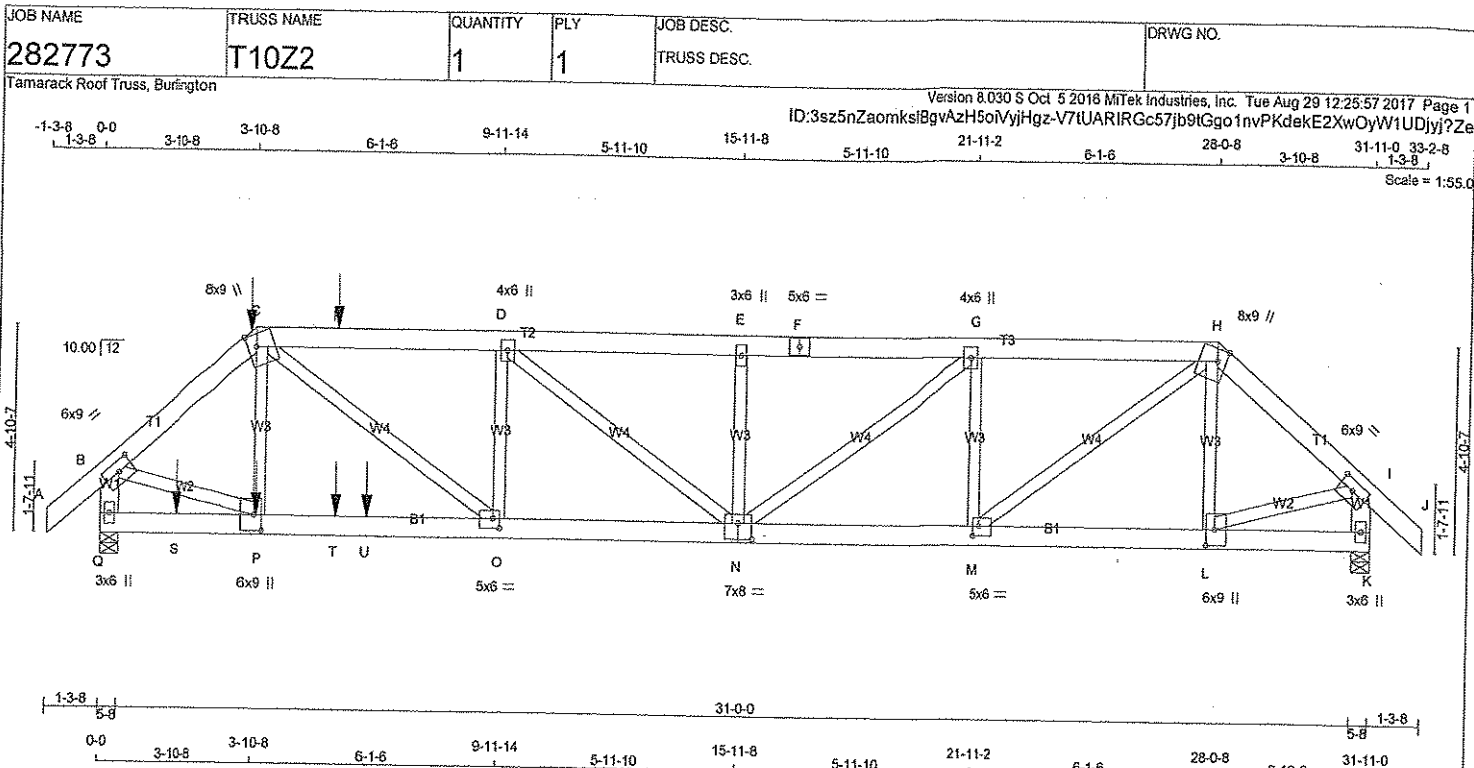
ISI GRIP= 0.88 (K) (INPUT = 0.90)
ISI METAL= 0.79 (K) (INPUT = 1.00)



DWG NO. TAM43557-17
STRUCTURAL
COMPONENT ONLY



DWG NO. TAM 43569-17
STRUCTURAL
COMPONENT ONLY



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x6	DRY	No.2
C - F	2x6	DRY	No.2
F - H	2x6	DRY	No.2
H - J	2x6	DRY	No.2
Q - B	2x6	DRY	No.2
K - I	2x6	DRY	No.2
Q - N	2x6	DRY	2100F 1.8E
N - K	2x6	DRY	2100F 1.8E

ALL WEBS 2x4 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-I	MT20	6.0	9.0	2.75	4.25
C	TTWW+m	MT20	8.0	9.0	3.50	2.50
D	TMVW+I	MT20	4.0	6.0		
E	TMVW+I	MT20	3.0	6.0		
F	TS-I	MT20	5.0	6.0		
G	TMVW+I	MT20	4.0	6.0		
H	TTWW+m	MT20	8.0	9.0	3.50	2.50
I	TMVW-I	MT20	6.0	9.0	2.75	4.25
K	BMV+I	MT20	3.0	6.0		
L	BMVW+I	MT20	6.0	9.0	4.50	2.50
M	BMVW+I	MT20	5.0	6.0	2.75	2.00
N	BSVW+I	MT20	7.0	8.0	4.50	4.00
O	BMVW+I	MT20	5.0	6.0	2.75	2.00
P	BMVW+I	MT20	6.0	9.0	4.50	2.50
Q	BMV+I	MT20	3.0	6.0		

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 225.2 lbs FACTORED DOWN AT 3-10-8, AND 38.3 lbs FACTORED DOWN AT 5-11-4 ON TOP CHORD, AND 42.0 lbs FACTORED DOWN AT 1-11-4, 34.1 lbs FACTORED DOWN AT 3-11-4, AND 34.1 lbs FACTORED DOWN AT 5-11-4, AND 1938.7 lbs FACTORED DOWN AT 6-8-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED		MAXIMUM FACTORED		INPUT		REQD	
	GROSS REACTION		GROSS REACTION		BRG		BRG	
Q	3727	0	3727	0	5-8		5-8	
K	2362	0	2362	0	5-8		5-8	

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN. COMPONENT REACTIONS		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
	COMBINED									
Q	2993	1787 / 0	609 / 0	0 / 0	0 / 0	0 / 0	0 / 0	617 / 0	0 / 0	0 / 0
K	1909	1108 / 0	403 / 0	0 / 0	0 / 0	0 / 0	0 / 0	399 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.44 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED	FACTORED		MEMB.	MAX. FACTORED	FACTORED	
	FORCE (LBS)	VERT. LOAD LC1 (PLF)	MAX. CSI (LC)		FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 39	-84.3 -84.3	0.07 (1)	10.00	P-C	0 / 631	0.12 (1)
B-C	-4271 / 0	-84.3 -84.3	0.22 (1)	4.00	C-O	0 / 2428	0.43 (1)
C-R	-5263 / 0	-84.3 -84.3	0.46 (1)	3.44	O-D	-252 / 65	0.06 (1)
R-D	-5263 / 0	-84.3 -84.3	0.46 (1)	3.44	D-N	-505 / 0	0.37 (1)
D-E	-4857 / 0	-84.3 -84.3	0.38 (1)	3.65	N-E	-467 / 0	0.11 (1)
E-F	-4857 / 0	-84.3 -84.3	0.39 (1)	3.64	N-G	0 / 1209	0.21 (1)
F-G	-4857 / 0	-84.3 -84.3	0.39 (1)	3.64	M-G	-1270 / 0	0.29 (1)
G-H	-3885 / 0	-84.3 -84.3	0.33 (1)	4.07	M-H	0 / 2551	0.45 (1)
H-I	-2383 / 0	-84.3 -84.3	0.15 (1)	5.17	L-H	-346 / 22	0.08 (1)
I-J	0 / 39	-84.3 -84.3	0.07 (1)	10.00	B-P	0 / 3390	0.60 (1)
Q-B	-3940 / 0	0.0	0.0	2.28 (1)	L-I	0 / 1892	0.33 (1)
K-I	-2325 / 0	0.0	0.0	0.17 (1)			
Q-S	0 / 0	-28.0	-28.0	0.45 (1)	10.00		
S-P	0 / 0	-28.0	-28.0	0.45 (1)	10.00		
P-T	0 / 3289	-28.0	-28.0	0.98 (1)	10.00		
T-U	0 / 3289	-28.0	-28.0	0.98 (1)	10.00		
U-O	0 / 3289	-28.0	-28.0	0.98 (1)	10.00		
O-N	0 / 5263	-28.0	-28.0	0.60 (1)	10.00		
N-M	0 / 3885	-28.0	-28.0	0.29 (1)	10.00		
M-L	0 / 1811	-28.0	-28.0	0.13 (2)	10.00		
L-K	0 / 0	-28.0	-28.0	0.05 (2)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.
C	3-10-8	-225	-225		BACK	VERT
P	3-11-4	-20	-34		BACK	VERT
R	5-11-4	-38	-38		BACK	VERT
S	1-11-4	-24	-42		BACK	VERT
T	5-11-4	-20	-34		BACK	VERT
U	6-8-8	-1937	-1937		BACK	VERT

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 25.6	PSF
	DL	= 3.0	PSF
BOT CH.	LL	= 10.5	PSF
	DL	= 7.0	PSF
TOTAL LOAD		= 46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (1.06")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.25")
ALLOWABLE DEFL.(TL) = $L/360$ (1.06")
CALCULATED VERT. DEFL.(TL) = $L/966$ (0.40")

CSI: TC=0.46 (C-D:1), BC=0.98 (O-P:1), WB=0.60 (B-P:1), SSI=0.81 (O-P:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

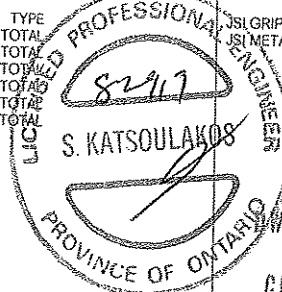
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	618	354	1687
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (M) (INPUT = 0.90)
JSI METAL= 0.82 (N) (INPUT = 1.00)

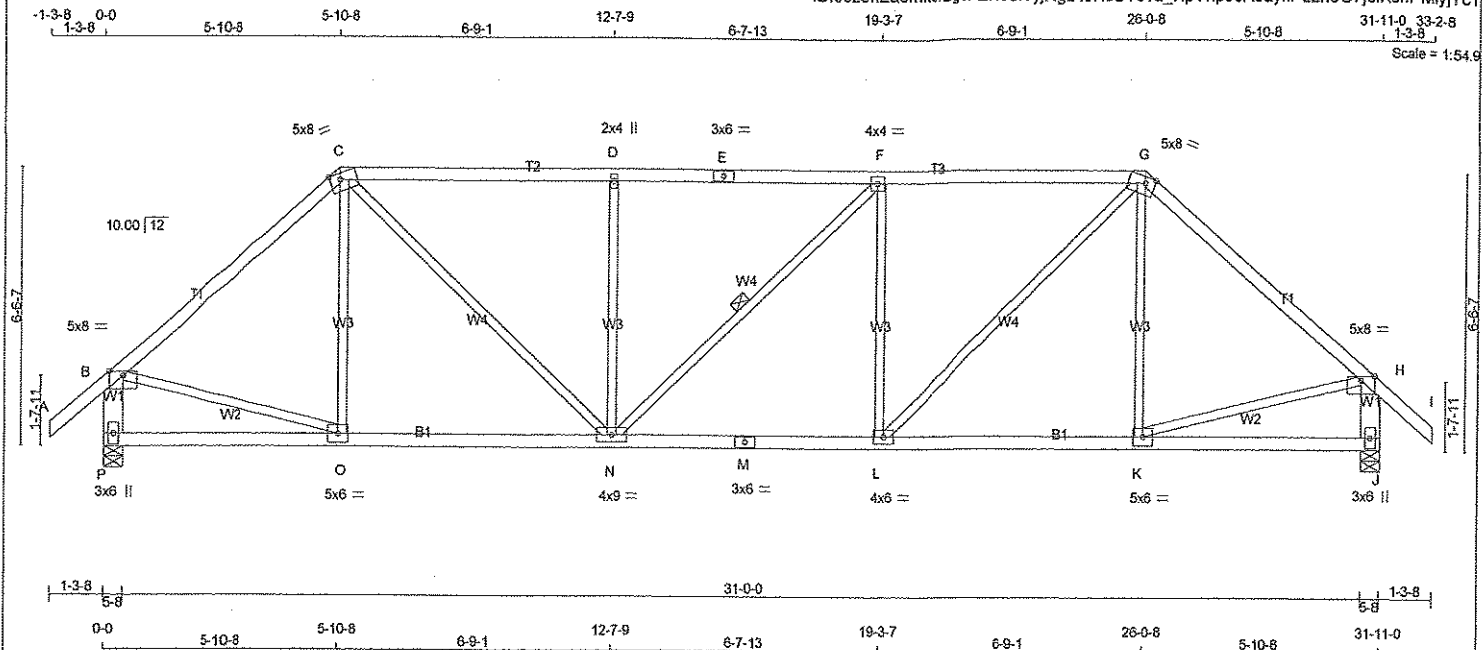


DRWG NO. TAM 43573-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
282771	T11	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 8 Oct 5 2016 MiTek Industries, Inc. Tue Aug 29 12:22:56 2017 Page 1
ID:3sz5nZaomkslBgvAzH5oiVvjHgZ-IJR3BY5vu_HpVnp59A3aykPLzh5O7jolKshPMiyj?cT



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - H	2x6	DRY	No.2
H - J	2x6	DRY	No.2
J - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
SPF			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMVV-p	MT20	5.0	8.0 Edge 3.00
C	TTWW-m	MT20	5.0	8.0 Edge 3.00
D	TMV-w	MT20	2.0	4.0
E	TS-t	MT20	3.0	6.0
F	TMVW-t	MT20	4.0	4.0
G	TTWW-m	MT20	5.0	8.0 Edge 3.00
H	TMVW-p	MT20	5.0	8.0 Edge
J	BMV1-p	MT20	3.0	6.0
K	BMVW-t	MT20	5.0	6.0
L	BMVW-t	MT20	4.0	6.0
M	BS-t	MT20	3.0	6.0
N	BMVW-t	MT20	4.0	9.0
O	BMVW-t	MT20	5.0	6.0
P	BMV1-p	MT20	3.0	6.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER							
BEARINGS							
JT	VERT	HORZ	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG	
P	1908	0	1908	0	5-8	5-8	
J	1908	0	1908	0	5-8	5-8	

UNFACTORED REACTIONS							
JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1550	887 / 0	335 / 0	0 / 0	0 / 0	327 / 0	0 / 0
J	1550	887 / 0	335 / 0	0 / 0	0 / 0	327 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 37	-84.3 -84.3	0.12 (1)	10.00	C-C	-107 / 188	0.07 (1)
B-C	-1828 / 0	-84.3 -84.3	0.71 (1)	4.19	C-N	0 / 1173	0.26 (1)
C-D	-2262 / 0	-84.3 -84.3	0.73 (1)	3.78	N-D	-611 / 0	0.43 (1)
D-E	-2262 / 0	-84.3 -84.3	0.73 (1)	3.77	N-F	-2 / 0	0.00 (1)
E-F	-2262 / 0	-84.3 -84.3	0.73 (1)	3.77	L-F	-612 / 0	0.43 (1)
F-G	-2263 / 0	-84.3 -84.3	0.74 (1)	3.77	L-G	0 / 1175	0.26 (1)
G-H	-1827 / 0	-84.3 -84.3	0.71 (1)	4.19	K-G	-108 / 188	0.08 (1)
H-I	0 / 37	-84.3 -84.3	0.12 (1)	10.00	B-O	0 / 1434	0.32 (1)
P-B	-1843 / 0	0.0 0.0	0.13 (1)	7.36	K-H	0 / 1434	0.32 (1)
J-H	-1843 / 0	0.0 0.0	0.13 (1)	7.36			
P-O	0 / 0	-28.0 -28.0	0.27 (3)	10.00			
O-N	0 / 1400	-28.0 -28.0	0.44 (2)	10.00			
N-M	0 / 2264	-28.0 -28.0	0.50 (2)	10.00			
M-L	0 / 2264	-28.0 -28.0	0.50 (2)	10.00			
L-K	0 / 1399	-28.0 -28.0	0.44 (2)	10.00			
K-J	0 / 0	-28.0 -28.0	0.27 (3)	10.00			

DESIGN CRITERIA	
SPECIFIED LOADS:	
TOP CH.	LL = 25.6 PSF
	DL = 3.0 PSF
BOT CH.	LL = 10.5 PSF
	DL = 7.0 PSF
TOTAL LOAD	= 46.1 PSF
SPACING = 24.0 IN. C/C	

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.06")
CALCULATED VERT. DEFL.(LL) = L/999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (1.06")
CALCULATED VERT. DEFL.(TL) = L/999 (0.22")

CSI: TC=0.74 (F-G:1), BC=0.50 (L-N:2), WB=0.43 (F-L:1), SSI=0.26 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

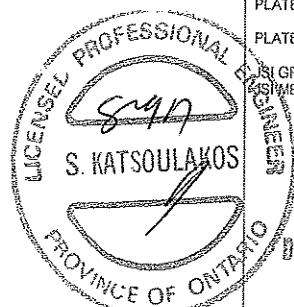
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (FSI) (PLI) (PLI)
MAX MIN MAX MIN
MT20 618 354 1687 822 2264 1656

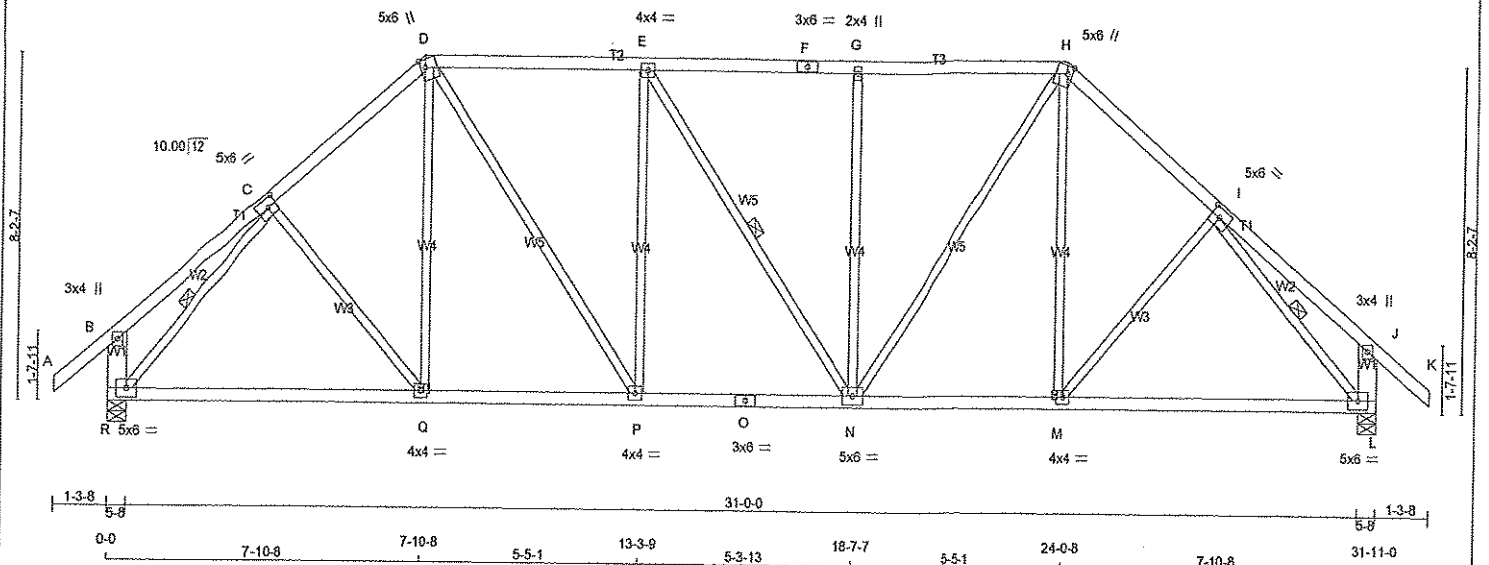
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

GRIP= 0.86 (G) (INPUT = 0.90)
METAL= 0.71 (M) (INPUT = 1.00)



DRWG NO. TAM 43559-17
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
R - B	2x6	DRY	No.2
L - J	2x6	DRY	No.2
R - O	2x4	DRY	No.2
O - L	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.75
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW+m	MT20	5.0	6.0	2.25	1.50
I	TMWW-t	MT20	5.0	6.0	2.50	2.75
J	TMV+p	MT20	3.0	4.0		
L	BMVW1-t	MT20	5.0	6.0		
M, P, Q						
M	BMWW-t	MT20	4.0	4.0		
N	BMWWW-t	MT20	5.0	6.0		
O	BS-t	MT20	3.0	6.0		
R	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REORD BRG
JT	VERT	DOWN	0	0
R	HORZ	UP/LIFT	0	0
L	HORZ	UP/LIFT	0	0

UNFACTORED REACTIONS

	1ST LCASE	MAX	MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE	PERM.LIVE
R	1550	887 / 0	335 / 0	0 / 0
L	1550	887 / 0	335 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.55 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N, C-R, I-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	FACTORED (PLF)	VERT. LOAD LC1 MAX	MEMB.	FORCE (LBS)	FACTORED (PLF)	VERT. LOAD LC1 MAX
FR-TO				FR-TO			
A-B	0 / 37	-84.3	-84.3 0.12 (1)	C-Q	0 / 133	0.03 (3)	10.00
B-C	0 / 23	-84.3	-84.3 0.20 (1)	Q-D	0 / 311	0.07 (2)	10.00
C-D	-1805 / 0	-84.3	-84.3 0.26 (1)	D-P	0 / 748	0.17 (1)	4.73
D-E	-1792 / 0	-84.3	-84.3 0.41 (1)	P-E	-488 / 0	0.64 (1)	4.55
E-F	-1791 / 0	-84.3	-84.3 0.41 (1)	E-N	-2 / 0	0.09 (1)	4.55
F-G	-1791 / 0	-84.3	-84.3 0.41 (1)	N-G	-487 / 0	0.64 (1)	4.57
G-H	-1791 / 0	-84.3	-84.3 0.40 (1)	H-N	0 / 746	0.17 (1)	4.73
H-I	-1805 / 0	-84.3	-84.3 0.26 (1)	M-H	0 / 312	0.07 (2)	10.00
I-J	0 / 23	-84.3	-84.3 0.20 (1)	M-I	0 / 133	0.03 (3)	10.00
J-K	0 / 37	-84.3	-84.3 0.12 (1)	R-C	-2069 / 0	0.51 (1)	7.81
R-B	-245 / 0	0.0	0.0 0.02 (1)	I-L	-2069 / 0	0.51 (1)	7.81
L-J	-245 / 0	0.0	0.0 0.02 (1)				
R-Q	0 / 1346	-28.0	-28.0 0.53 (2)				
Q-P	0 / 1388	-28.0	-28.0 0.55 (2)				
P-O	0 / 1792	-28.0	-28.0 0.37 (1)				
O-N	0 / 1792	-28.0	-28.0 0.37 (1)				
N-M	0 / 1388	-28.0	-28.0 0.55 (2)				
M-L	0 / 1346	-28.0	-28.0 0.53 (2)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 3.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 46.1	PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.06")
CALCULATED VERT. DEFL.(LL) = L/999 (0.17")
ALLOWABLE DEFL.(TL) = L/360 (1.06")
CALCULATED VERT. DEFL.(TL) = L/999 (0.29")

CSI: TC=0.41 (D-E:1), BC=0.55 (M-N:2), WB=0.64 (E-P:1), SSI=0.21 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

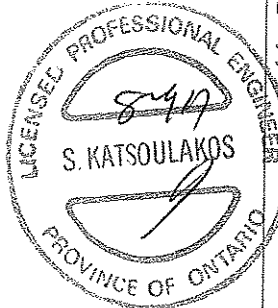
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
618	354	1687	822 2284 1656

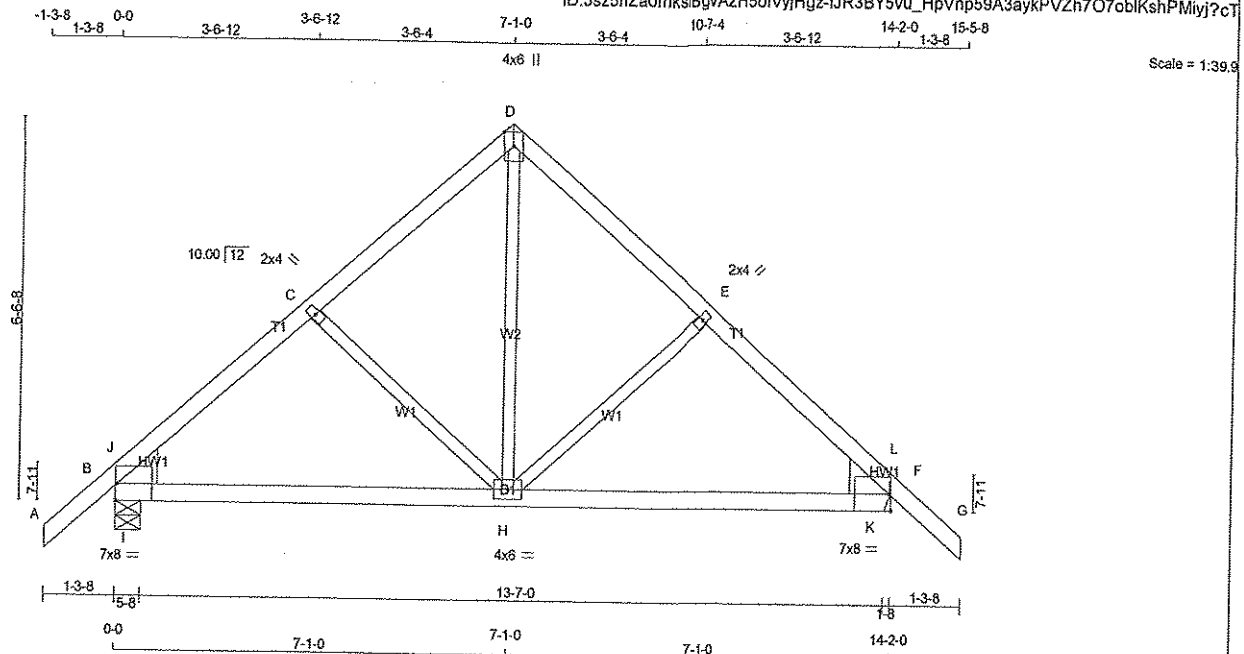
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (I) (INPUT = 0.90)
JSI METAL = 0.58 (O) (INPUT = 1.00)



DRWG NO. TAM 4356017
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-I	MT20	7.0	8.0		Edge
C	TMW+w	MT20	2.0	4.0		
D	TTW+p	MT20	4.0	6.0		Edge
E	TMW+w	MT20	2.0	4.0		
F	TMBH1-I	MT20	7.0	8.0		Edge
H	BMWWW-I	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG	HEEL WEDGE
	VERT	HORZ	DOWN	HORZ			
B	912	0	912	0	5-8	5-8	2x8 L
F	912	0	912	0	5-8	5-8	2x8 R

HANGER BY OTHERS MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	732	433 / 0	149 / 0	0 / 0	0 / 0	150 / 0	0 / 0
F	732	433 / 0	149 / 0	0 / 0	0 / 0	150 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.68 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	UNBRACED LENGTH FR-TO
A-B	0 / 30	-84.3	-84.3 0.11 (1)	H-D	0 / 536	0.12 (1)	10.00
B-J	-1269 / 0	-84.3	-84.3 0.08 (3)	H-E	-257 / 0	0.10 (1)	5.68
J-C	-853 / 0	-84.3	-84.3 0.11 (1)	C-H	-257 / 0	0.10 (1)	6.25
C-D	-631 / 0	-84.3	-84.3 0.12 (1)	I-J	0 / 584	0.00 (1)	6.25
D-E	-631 / 0	-84.3	-84.3 0.12 (1)	K-L	0 / 584	0.00 (1)	6.25
E-L	-853 / 0	-84.3	-84.3 0.11 (1)				5.68
L-F	-1269 / 0	-84.3	-84.3 0.08 (3)				10.00
F-G	0 / 30	-84.3	-84.3 0.11 (1)				
B-I	0 / 683	-28.0	-28.0 0.24 (2)				
I-H	0 / 683	-28.0	-28.0 0.37 (2)				
H-K	0 / 683	-28.0	-28.0 0.37 (2)				
K-F	0 / 683	-28.0	-28.0 0.24 (2)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.47")
CALCULATED VERT. DEFL. (LL) = L/999 (0.05")
ALLOWABLE DEFL. (TL) = L/360 (0.47")
CALCULATED VERT. DEFL. (TL) = L/999 (0.07")

CSI: TC=0.12 (D-E:1), BC=0.37 (H-K:2), WB=0.12 (D-H:1), SSI=0.34 (F-K:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

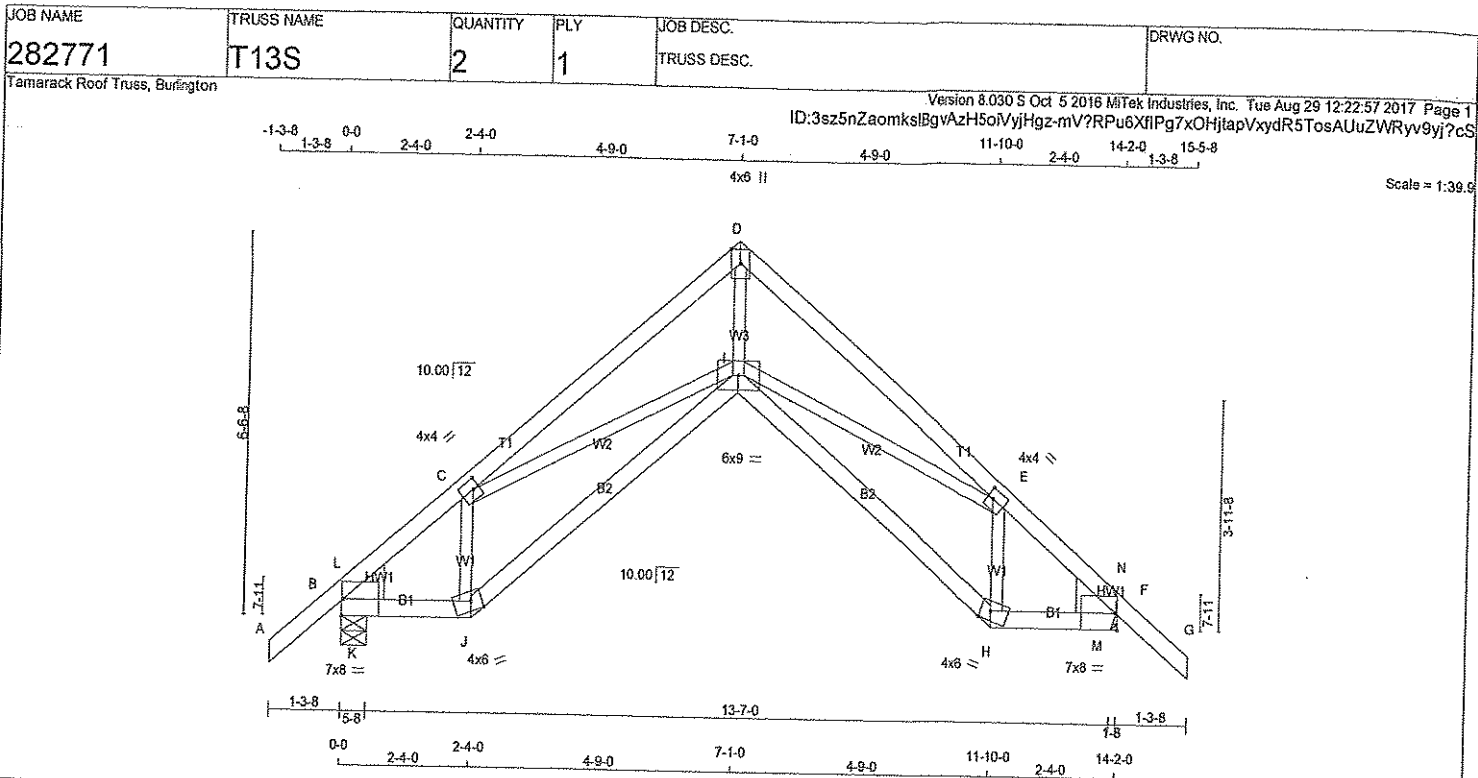
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (F) (INPUT = 0.90)
JSI METAL = 0.19 (F) (INPUT = 1.00)



DRWG NO. TAM 4356/-17
STRUCTURAL
COMPONENT ONLY

TOTAL WEIGHT = 57 lb
(M/J/F)



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2 SPF
D - G	2x4	DRY	No.2 SPF
B - J	2x4	DRY	No.2 SPF
J - I	2x4	DRY	No.2 SPF
I - H	2x4	DRY	No.2 SPF
H - F	2x4	DRY	No.2 SPF
ALL WEBS	2x3	DRY	No.2 SPF
DRY: SEASONED LUMBER.			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-I	MT20	7.0	8.0		Edge
C	TMWW-I	MT20	4.0	4.0	2.00	1.25
D	TTW+p	MT20	4.0	6.0		Edge
E	TMWW-I	MT20	4.0	4.0	2.00	1.25
F	TMBH1-I	MT20	7.0	8.0		Edge
H	BBW-m	MT20	4.0	6.0	2.00	2.25
I	BBWWW-p	MT20	6.0	9.0		
J	BBW-m	MT20	4.0	6.0	2.00	2.25

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG	HEEL
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
B	912	0	912	0	0
F	912	0	912	0	0

HANGER BY OTHERS MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN COMPONENT REACTIONS	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
JT	COMBINED							
B	732	433 / 0	149 / 0	0 / 0	0 / 0	150 / 0	0 / 0	0 / 0
F	732	433 / 0	149 / 0	0 / 0	0 / 0	150 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.83 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)
FR-TO		FROM TO	FR-TO	FROM TO
A-B	0 / 30	-84.3 -84.3 0.11 (1)	I-D	0 / 1776 0.40 (1)
B-L	-1423 / 0	-84.3 -84.3 0.10 (1)	J-E	0 / 594 0.13 (1)
L-C	-988 / 0	-84.3 -84.3 0.18 (1)	H-E	-565 / 0 0.09 (1)
C-D	-1683 / 0	-84.3 -84.3 0.31 (1)	C-I	0 / 594 0.13 (1)
D-E	-1683 / 0	-84.3 -84.3 0.31 (1)	J-C	-565 / 0 0.09 (1)
E-N	-988 / 0	-84.3 -84.3 0.18 (1)	K-L	0 / 502 0.00 (1)
N-F	-1423 / 0	-84.3 -84.3 0.10 (1)	M-N	0 / 502 0.00 (1)
F-G	0 / 30	-84.3 -84.3 0.11 (1)		
B-K	0 / 781	-28.0 -28.0 0.29 (1)		
K-J	0 / 781	-28.0 -28.0 0.29 (1)		
J-I	0 / 943	-28.0 -28.0 0.29 (2)		
I-H	0 / 943	-28.0 -28.0 0.29 (2)		
H-M	0 / 781	-28.0 -28.0 0.29 (1)		
M-F	0 / 781	-28.0 -28.0 0.29 (1)		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 3.0 PSF
BOT CH. LL	= 10.5 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.47")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.09")
ALLOWABLE DEFL. (TL) = $L/360$ (0.47")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.15")

CSI: TC=0.31 (D-E:1), BC=0.29 (I-J:2), WB=0.40 (D-I:1), SSI=0.33 (F-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
MT20	618	354	1657
	822	2284	1656

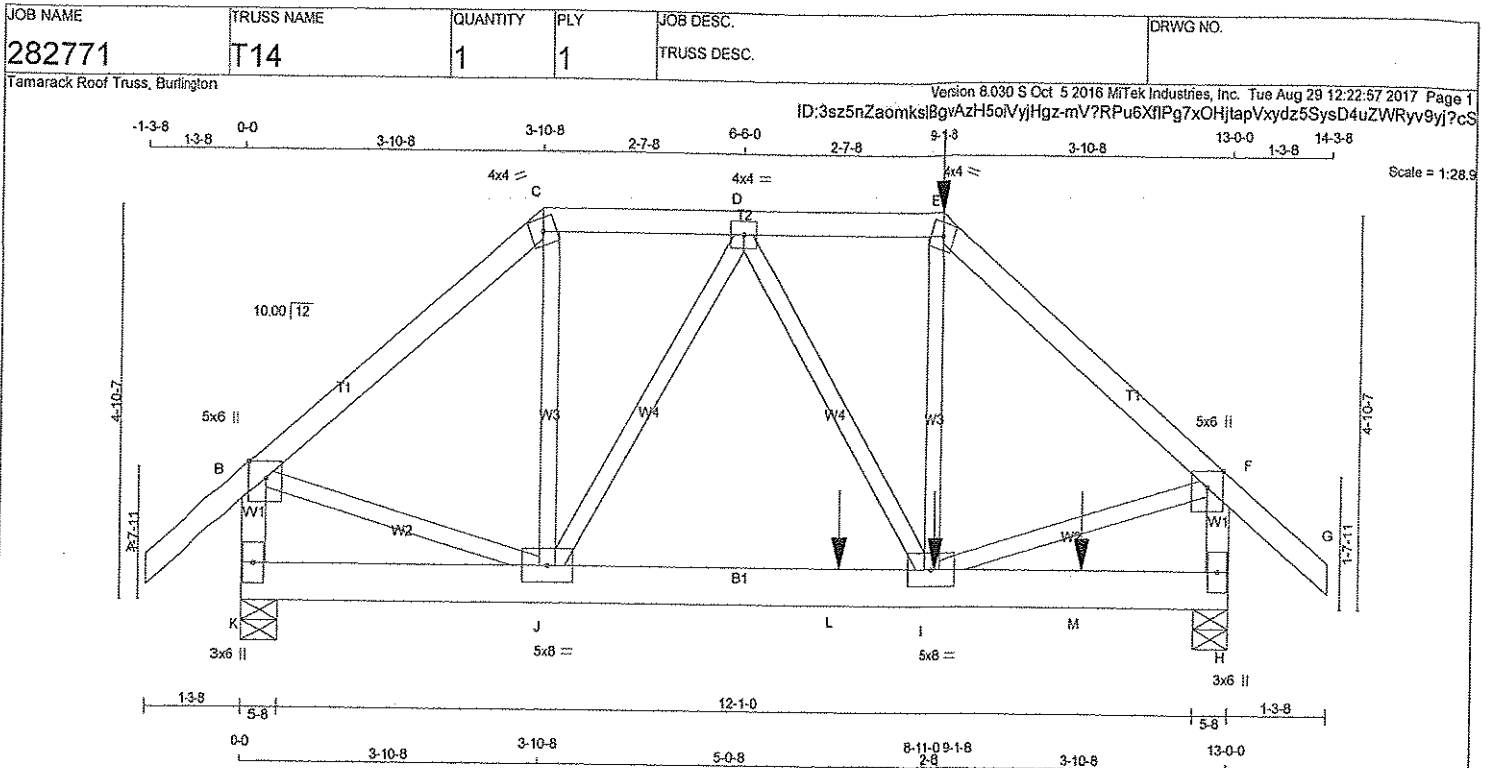
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (I) (INPUT = 0.90)
JSI METAL= 0.33 (C) (INPUT = 1.00)



DRWG NO. TAM43562-17
STRUCTURAL
COMPONENT ONLY



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
K - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
K - H	2x6	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTW-m	MT20	4.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TTW-m	MT20	4.0	4.0		
F	TMVW+p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	6.0		
I	BMWWW-t	MT20	5.0	8.0		
J	BMWWW-t	MT20	5.0	8.0		
K	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 225.2 lbs FACTORED DOWN AT 9-1-8 ON TOP CHORD, AND 483.6 lbs FACTORED DOWN AT 7-9-8, AND 34.1 lbs FACTORED DOWN AT 9-0-12, AND 42.0 lbs FACTORED DOWN AT 11-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
K	1117	0	1117	0	5-8	5-8
H	1328	0	1328	0	5-8	5-8

UNFACTORED REACTIONS							
	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	885	542 / 0	167 / 0	0 / 0	0 / 0	176 / 0	0 / 0
H	1051	645 / 0	198 / 0	0 / 0	0 / 0	209 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.52 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRAC LENGTH FR-TO	MAX. FACTORED FORCE (LBS)
FR-TO				FR-TO			
A-B	0 / 37	-84.3	-84.3 0.13 (1)	J-C	0 / 341	10.00	0.08 (1)
B-C	-930 / 0	-84.3	-84.3 0.26 (1)	I-E	0 / 318	6.05	0.08 (2)
C-D	-720 / 0	-84.3	-84.3 0.10 (1)	B-J	0 / 745	6.25	0.18 (1)
D-E	-909 / 0	-84.3	-84.3 0.10 (1)	I-F	0 / 944	6.25	0.23 (1)
E-F	-1178 / 0	-84.3	-84.3 0.27 (1)	J-D	-338 / 0	5.52	0.15 (1)
F-G	0 / 37	-84.3	-84.3 0.13 (1)	D-I	0 / 64	10.00	0.02 (2)
K-B	-1097 / 0	0.0	0.0 0.13 (1)			7.52	
H-F	-1315 / 0	0.0	0.0 0.15 (1)			7.02	
K-J	0 / 0	-28.0	-28.0 0.11 (2)			10.00	
J-L	0 / 886	-28.0	-28.0 0.35 (1)			10.00	
L-I	0 / 886	-28.0	-28.0 0.35 (1)			10.00	
I-M	0 / 0	-28.0	-28.0 0.16 (1)			10.00	
M-H	0 / 0	-28.0	-28.0 0.16 (1)			10.00	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
E	9-1-8	-225	-225	---	BACK	VERT	TOTAL
I	9-0-12	-20	-34	---	BACK	VERT	TOTAL
L	7-9-8	-484	-484	---	BACK	VERT	TOTAL
M	11-0-12	-24	-42	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.27 (E-F:1), BC=0.35 (I-J:1), WB=0.23 (F-I:1), SSI=0.30 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

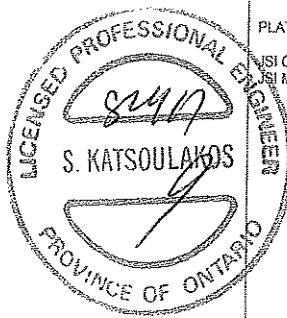
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 622 2284 1656

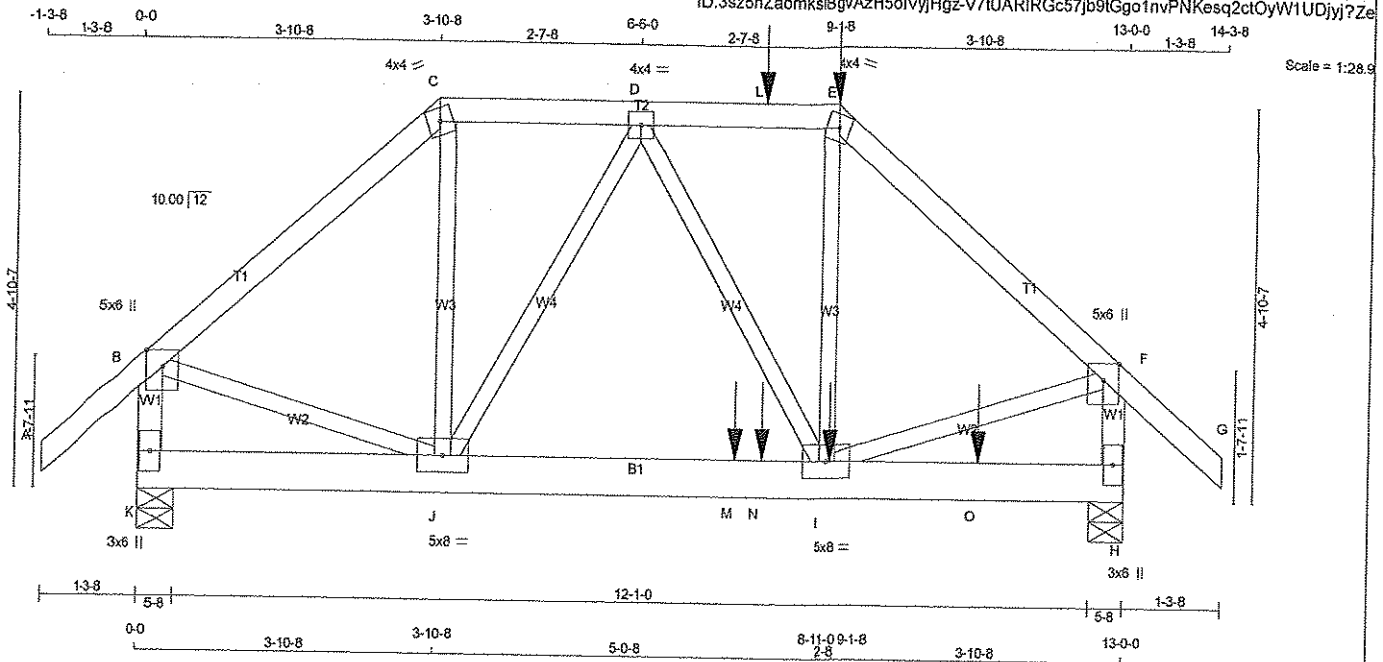
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SSI GRIP= 0.71 (J) (INPUT = 0.90)
SSI METAL= 0.23 (F) (INPUT = 1.00)



DWG NO. TAM 43563-17
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 68 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
K - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
K - H	2x6	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMVW+p	MT20	5.0	6.0 Edge
C	TTW-m	MT20	4.0	4.0
D	TMWW-t	MT20	4.0	4.0
E	TTW-m	MT20	4.0	4.0
F	TMVW+p	MT20	5.0	6.0 Edge
H	BMV1+p	MT20	3.0	6.0
I	BMWW-t	MT20	5.0	8.0
J	BMWW-t	MT20	5.0	8.0
K	BMV1+p	MT20	3.0	6.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 39.2 lbs FACTORED DOWN AT 8-1-12, AND 225.2 lbs FACTORED DOWN AT 9-1-8 ON TOP CHORD, AND 744.2 lbs FACTORED DOWN AT 7-9-8, 34.1 lbs FACTORED DOWN AT 8-1-12, AND 34.1 lbs FACTORED DOWN AT 9-0-12, AND 42.0 lbs FACTORED DOWN AT 11-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORIZ	DOWN	UP
K	1243	0	1243	0
H	1521	0	1521	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN	COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
K	983	606 / 0	183 / 0	0 / 0	0 / 0	194 / 0	0 / 0
H	1202	743 / 0	222 / 0	0 / 0	0 / 0	237 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.11 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 37	-84.3 -84.3	0.13 (1)	10.00	J-C	0 / 445	0.11 (1)
B-C	-1090 / 0	-84.3 -84.3	0.27 (1)	5.69	I-E	0 / 409	0.10 (1)
C-D	-845 / 0	-84.3 -84.3	0.11 (1)	6.25	B-J	0 / 874	0.22 (1)
D-L	-1102 / 0	-84.3 -84.3	0.12 (1)	5.87	I-F	0 / 1143	0.28 (1)
L-E	-1102 / 0	-84.3 -84.3	0.12 (1)	5.11	J-D	-417 / 0	0.18 (1)
E-F	-1426 / 0	-84.3 -84.3	0.29 (1)	5.11	D-I	0 / 108	0.03 (1)
F-G	0 / 37	-84.3 -84.3	0.13 (1)	10.00			
G-H	-1237 / 0	0.0 0.0	0.14 (1)	7.19			
H-F	-1532 / 0	0.0 0.0	0.17 (1)	6.81			
K-J	0 / 0	-28.0 -28.0	0.15 (1)	10.00			
J-M	0 / 1049	-28.0 -28.0	0.49 (1)	10.00			
M-N	0 / 1049	-28.0 -28.0	0.49 (1)	10.00			
N-I	0 / 1049	-28.0 -28.0	0.49 (1)	10.00			
I-O	0 / 0	-28.0 -28.0	0.23 (1)	10.00			
O-H	0 / 0	-28.0 -28.0	0.23 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
E	9-1-8	-225	-225	-	BACK	VERT	TOTAL
I	9-0-12	-20	-34	-	BACK	VERT	TOTAL
L	8-1-12	-39	-39	-	BACK	VERT	TOTAL
M	7-9-8	-744	-744	-	BACK	VERT	TOTAL
N	8-1-12	-20	-34	-	BACK	VERT	TOTAL
O	11-0-12	-24	-42	-	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.43")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.05")
ALLOWABLE DEFL.(TL) = $L/360$ (0.43")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.07")

CSI: TC=0.29 (E-F:1), BC=0.49 (I-J:1), WB=0.28 (F-I:1), SSI=0.45 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00

COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

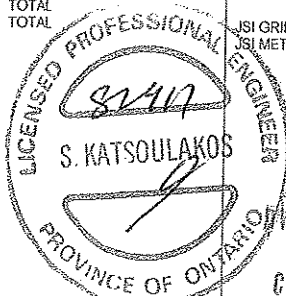
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

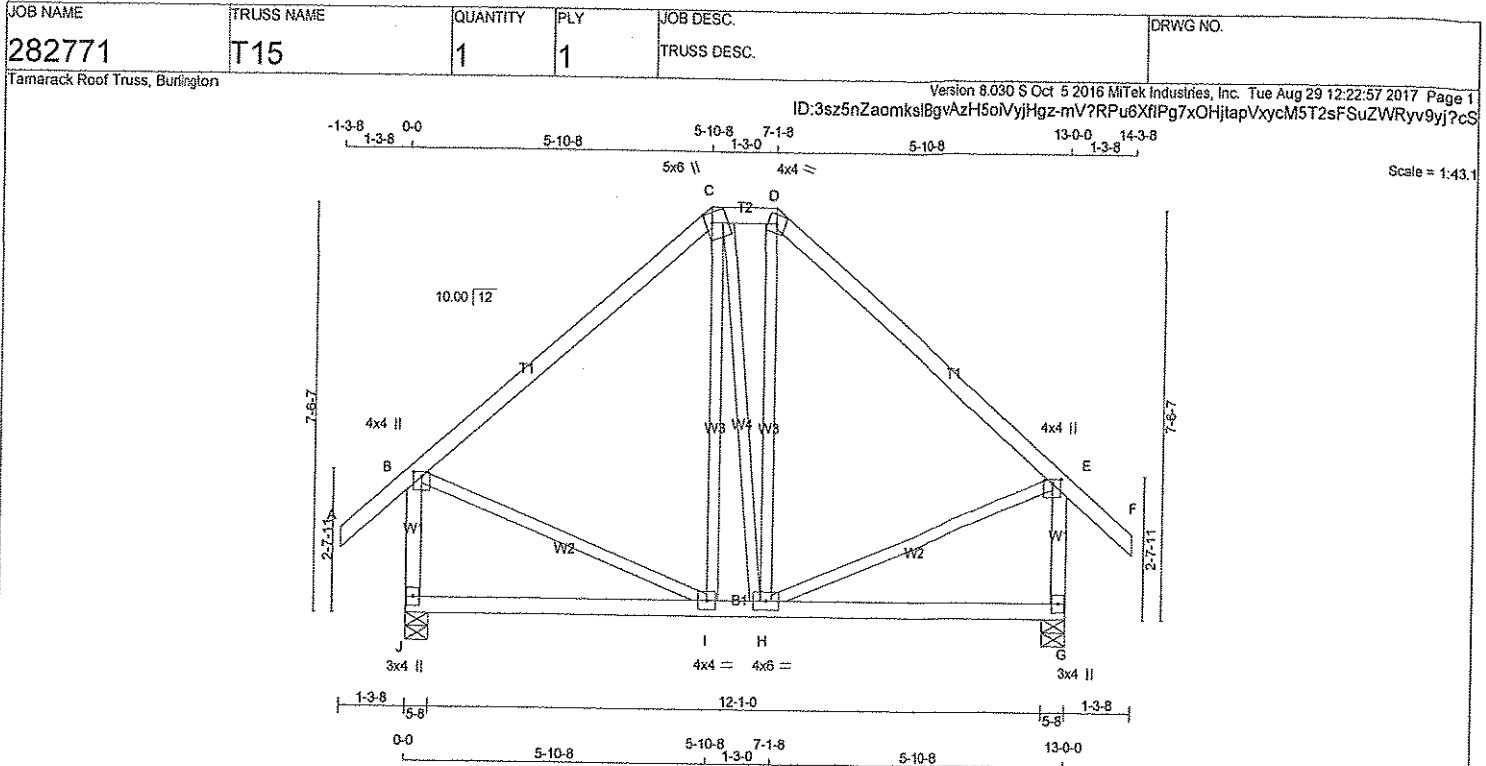
PLATE PLACEMENT TOL. = 0.250 inches

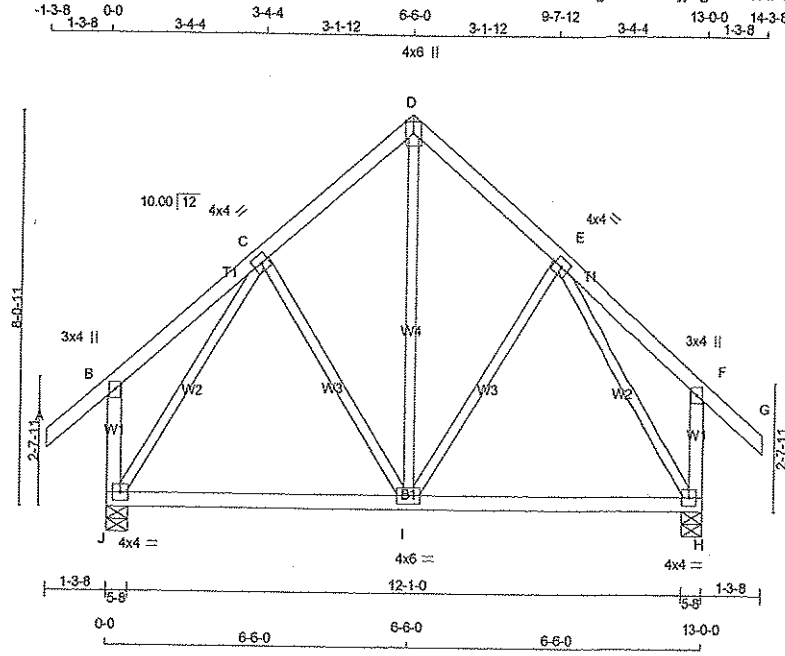
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (J) (INPUT = 0.90)
JSI METAL= 0.27 (F) (INPUT = 1.00)



DRWG NO. TAM 43574-17
STRUCTURAL
COMPONENT ONLY





TOTAL WEIGHT = 3 X 68 = 204 lb [M/JF]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2 SPF
D - G	2x4	DRY	No.2 SPF
J - B	2x4	DRY	No.2 SPF
H - F	2x4	DRY	No.2 SPF
J - H	2x4	DRY	No.2 SPF
ALL WEBS EXCEPT	2x3	DRY	No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-I	MT20	4.0	4.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-I	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW-I	MT20	4.0	4.0		
I	BMVW-I	MT20	4.0	6.0		
J	BMVW-I	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
J	846	0	846	0	5-8	5-8
H	846	0	846	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
J	678	403 / 0	136 / 0	0 / 0	0 / 0	136 / 0	0 / 0
H	678	403 / 0	136 / 0	0 / 0	0 / 0	136 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 MAX. (LC) UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LC1 MAX. (LC) UNBRACED LENGTH	
FR-TO				FR-TO			
A-B	0 / 37	-84.3	-84.3 0.12 (1)	10.00	I-D	0 / 345	0.08 (2)
B-C	0 / 20	-84.3	-84.3 0.14 (1)	10.00	I-E	-84 / 63	0.04 (1)
C-D	-444 / 0	-84.3	-84.3 0.11 (1)	6.25	C-I	-84 / 63	0.04 (1)
D-E	-444 / 0	-84.3	-84.3 0.11 (1)	6.25	J-C	-860 / 0	0.39 (1)
E-F	0 / 20	-84.3	-84.3 0.14 (1)	10.00	E-H	-860 / 0	0.39 (1)
F-G	0 / 37	-84.3	-84.3 0.12 (1)	10.00			
J-B	-224 / 0	0.0	0.0 0.03 (1)	7.81			
H-F	-224 / 0	0.0	0.0 0.03 (1)	7.81			
J-I	0 / 360	-28.0	-28.0 0.38 (2)	10.00			
I-H	0 / 360	-28.0	-28.0 0.38 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = 1/360 (0.43")
CALCULATED VERT. DEFL. (LL) = 1/999 (0.05")
ALLOWABLE DEFL. (TL) = 1/360 (0.43")
CALCULATED VERT. DEFL. (TL) = 1/999 (0.09")

CSI: TC=0.14 (E-F-I), BC=0.38 (H-I-2), WB=0.39 (E-H-I), SSI=0.15 (H-I-3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

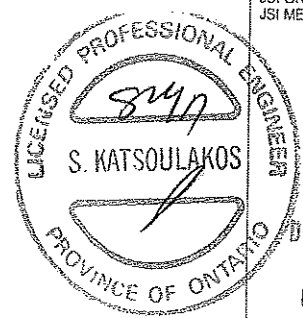
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PL)
MT20	618	354	1687 822 2204 1856

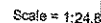
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (C) (INPUT = 0.90)
JSI METAL= 0.24 (E) (INPUT = 1.00)



DRWG NO. TAM 43565-17
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 43 lb

SPECIFIED LOADS:			
TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD =		46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 6.00/12

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

TOTAL LOAD CASES: (4)

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 134.2 lbs FACTORED DOWN AT 2-10,8, AND 134.2 lbs FACTORED DOWN AT 4-8,8 ON TOP CHORD, AND 19.5 lbs FACTORED DOWN AT 1-11,4, 14.0 lbs FACTORED DOWN AT 2-11,4, AND 14.0 lbs FACTORED DOWN AT 4-8,12, AND 19.5 lbs FACTORED DOWN AT 5-8,12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED			
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 37	-84.3	-84.3 0.13 (1)	10.00	J-E	-23 / 82	0.02 (3)
B-C	-432 / 0	-84.3	-84.3 0.13 (1)	6.25	I-C	-16 / 60	0.02 (3)
C-D	-330 / 0	-84.3	-84.3 0.01 (1)	6.25	B-J	0 / 361	0.09 (1)
D-E	-330 / 0	-84.3	-84.3 0.01 (3)	6.25	I-F	0 / 361	0.09 (1)
E-F	-432 / 0	-84.3	-84.3 0.13 (1)	6.25	J-D	-36 / 8	0.01 (1)
F-G	0 / 37	-84.3	-84.3 0.13 (1)	10.00	D-I	-43 / 10	0.01 (1)
K-B	-661 / 0	0.0	0.0 0.08 (1)	7.81			
H-F	-661 / 0	0.0	0.0 0.08 (1)	7.81			
K-L	0 / 0	-28.0	-28.0 0.07 (2)	10.00			
L-J	0 / 0	-28.0	-28.0 0.07 (2)	10.00			
J-I	0 / 339	-28.0	-28.0 0.10 (2)	10.00			
I-M	0 / 0	-28.0	-28.0 0.07 (2)	10.00			
M-H	0 / 0	-28.0	-28.0 0.07 (2)	10.00			

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= $L/360$ (0.26") :
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.01")
ALLOWABLE DEFL.(TL)= $L/360$ (0.26")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.01")

CSI: TC=0.13 (E-F:1), BC=0.10 (I-J:2), WB=0.09 (F-I:1), SS=0.09 (J-K:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

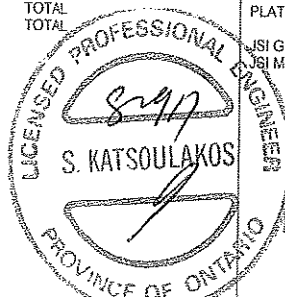
COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP(DRY)		SHEAR		SECTION
	(PSI)		(PLI)		(PLI)
	MAX	MIN	MAX	MIN	MAX MIN
MT20	618	354	1667	822	2284 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.
 JSI GRIP= 0.56 (B) (INPUT = 0.90)
 JSI METAL= 0.15 (B) (INPUT = 1.00)



QND NO. TAM 43566-17
STRUCTURAL
COMPONENT ONLY



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

TOTAL WEIGHT = 26 lb

UNFACTORED REACTIONS

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B. D.

BRACING

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED

LOADING

TOTAL LOAD CASES: (4)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= $L/360$ (0.26")
 CALCULATED VERT. DEFL.(LL) = $L/999$ (0.01")
 ALLOWABLE DEFL.(TL)= $L/360$ (0.26")
 CALCULATED VERT. DEFL.(TL) = $L/999$ (0.01")

CSI: TC=0.12 (C-J:1), BC=0.15 (F-G:1), WB=0.05 (C-F:2), SSI=0.11 (C-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

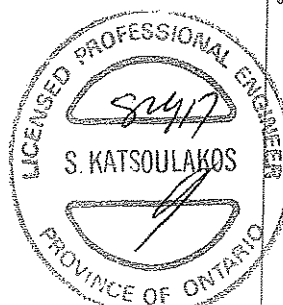
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1657	822	2284	1656

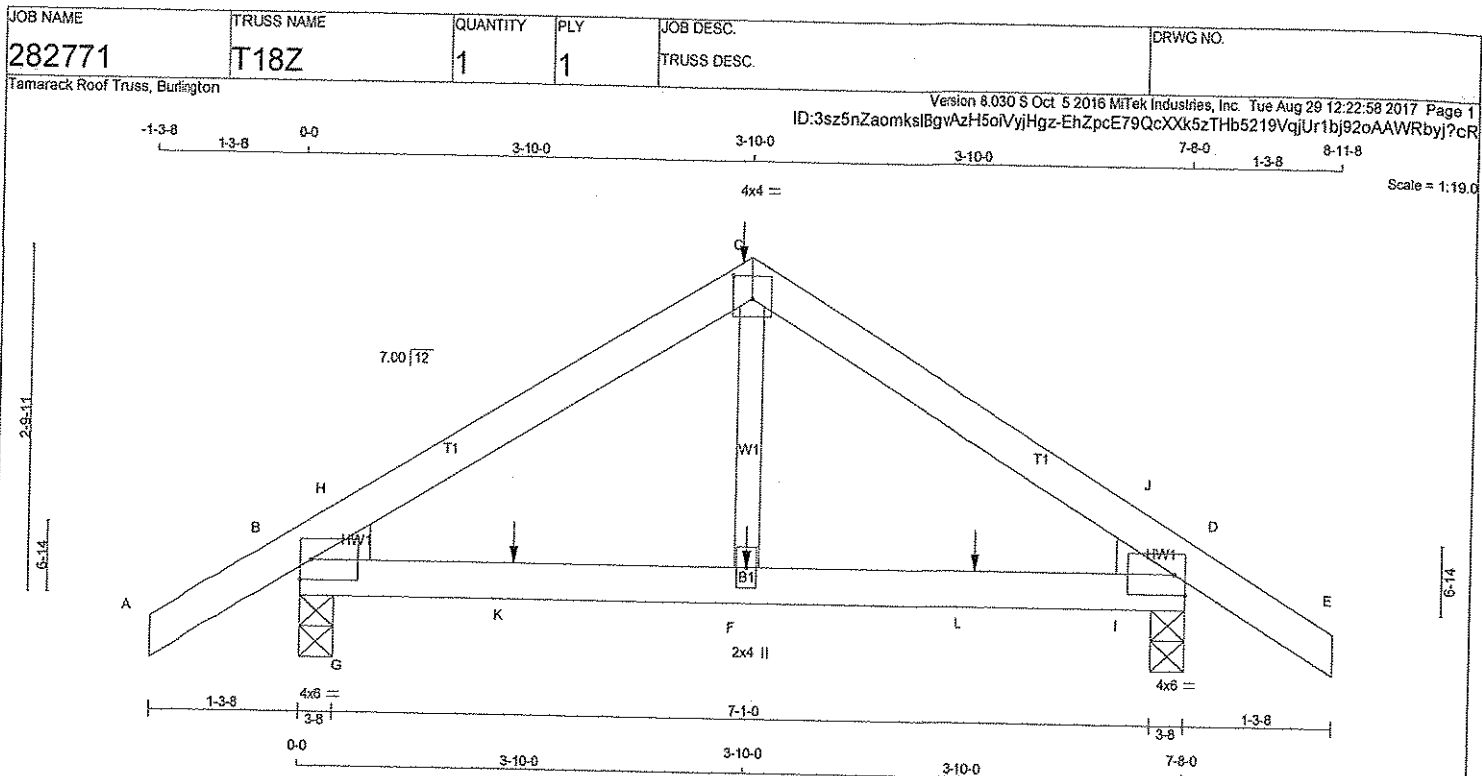
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg

ISI GRIP= 0.72 (B) (INPUT = 0.90)
ISI METAL= 0.12 (B) (INPUT = 1.00)



DWG NO. TAM 43567-17
STRUCTURAL
COMPONENT ONLY



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
B - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2

DRY, SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-I	MT20	4.0	6.0		Edge
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMBH1-I	MT20	4.0	6.0		Edge
F	BMW+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) AT 3-10-0 ON TOP CHORD, AND AT 1-10-0, AND AT 3-10-0, AND AT 5-10-0 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
	VERT	HORZ	DOWN	HORZ			
B	545	0	545	0	3-8	3-8	2x4 L
D	545	0	545	0	3-8	3-8	2x4 R

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	431	286 / 0	81 / 0	0 / 0	0 / 0	85 / 0	0 / 0
D	431	286 / 0	81 / 0	0 / 0	0 / 0	85 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 MAX. (LBS)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 MAX. (LBS)
FR-TO		FROM TO			FR-TO		
A-B	0 / 21	-84.3 -84.3	0.12 (1)	10.00	F-C	0 / 211	0.05 (2)
B-H	-454 / 0	-84.3 -84.3	0.09 (1)	6.25	G-H	-98 / 113	0.00 (1)
H-C	-423 / 0	-84.3 -84.3	0.15 (1)	6.25	I-J	-98 / 113	0.00 (1)
C-J	-423 / 0	-84.3 -84.3	0.15 (1)	6.25			
J-D	-454 / 0	-84.3 -84.3	0.09 (1)	6.25			
D-E	0 / 21	-84.3 -84.3	0.12 (1)	10.00			
B-G	0 / 358	-28.0 -28.0	0.14 (1)	10.00			
G-K	0 / 358	-28.0 -28.0	0.17 (1)	10.00			
K-F	0 / 358	-28.0 -28.0	0.17 (1)	10.00			
F-L	0 / 358	-28.0 -28.0	0.17 (1)	10.00			
L-I	0 / 358	-28.0 -28.0	0.17 (1)	10.00			
I-D	0 / 358	-28.0 -28.0	0.14 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE
C	3-10-0	---	---	---	BACK	VERT	TOTAL
F	3-10-0	---	---	---	BACK	VERT	TOTAL
K	1-10-0	---	---	---	BACK	VERT	TOTAL
L	5-10-0	---	---	---	BACK	VERT	TOTAL

TOTAL WEIGHT = 26 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.26")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.26")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.15 (C-J:1), BC=0.17 (F-G:1), WB=0.05 (C-F:2), SS=0.12 (C-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

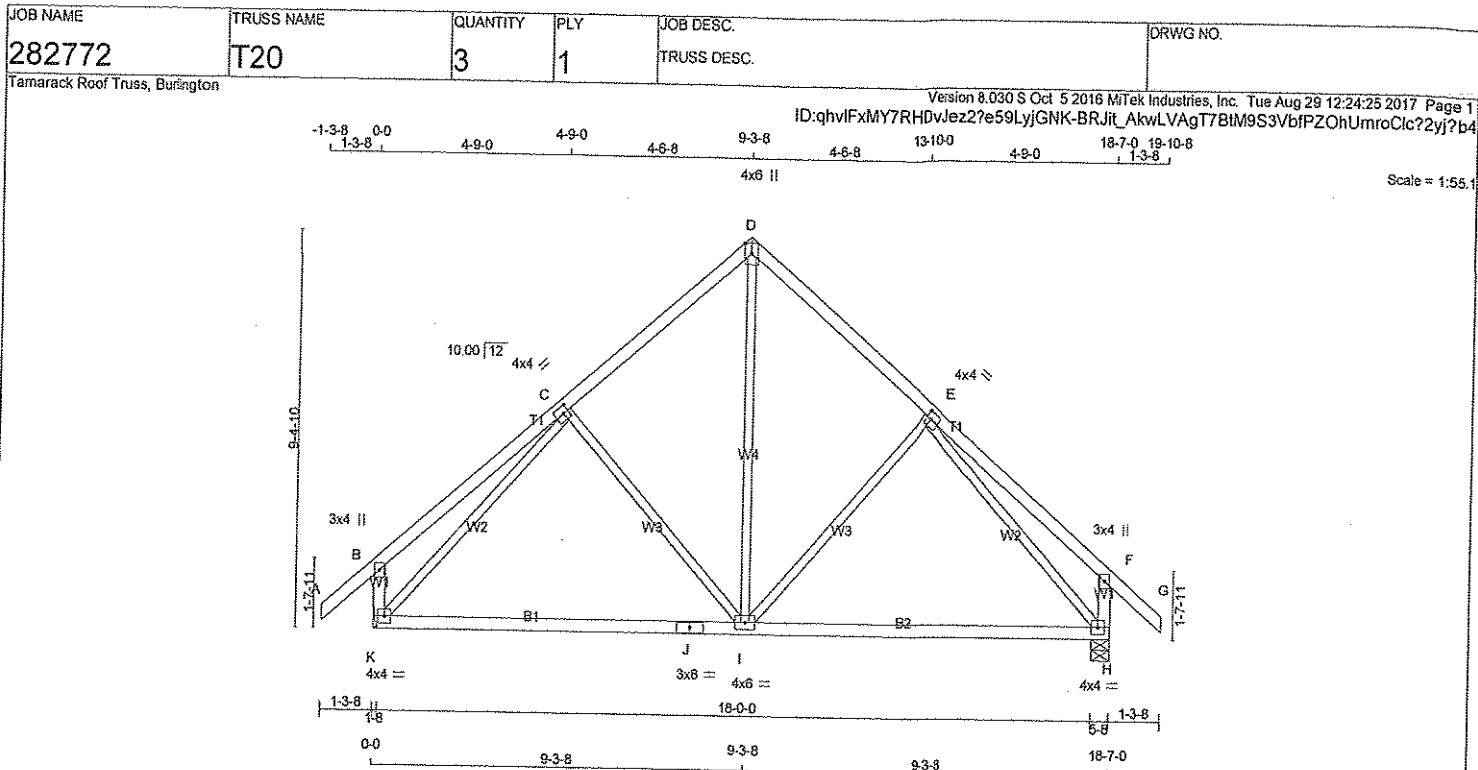
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.72 (B) (INPUT = 0.90)
JSI METAL= 0.12 (B) (INPUT = 1.00)



DRWG NO. TAM4356B17
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
K - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
K - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-l	MT20	4.0	4.0	2.00	1.50
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-l	MT20	4.0	4.0	2.00	1.50
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-l	MT20	4.0	4.0		
I	BMVWW1-l	MT20	4.0	6.0		
J	BS-l	MT20	3.0	6.0		
K	BMVW1-l	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
K	1160	0	1160	0	0	IN-SX
H	1160	0	1160	0	0	MIN. SEAT SIZE: 1-8 5-8

UNFACTORED REACTIONS

JT	COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
K	936	546 / 0	195 / 0	0 / 0	0 / 0	194 / 0	0 / 0
H	935	546 / 0	195 / 0	0 / 0	0 / 0	194 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. FACTORED CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 37	-84.3	-84.3 0.12 (1)	10.00	I-D	0 / 845	0.15 (1)
B-C	0 / 30	-84.3	-84.3 0.30 (1)	10.00	I-E	-217 / 35	0.20 (1)
C-D	-769 / 0	-84.3	-84.3 0.24 (1)	6.25	C-I	-217 / 35	0.20 (1)
D-E	-769 / 0	-84.3	-84.3 0.24 (1)	6.25	K-C	-1070 / 0	0.94 (1)
E-F	0 / 30	-84.3	-84.3 0.30 (1)	10.00	E-H	-1070 / 0	0.94 (1)
F-G	0 / 37	-84.3	-84.3 0.12 (1)	10.00			
K-B	-265 / 0	0.0	0.0 0.03 (1)	7.81			
H-F	-265 / 0	0.0	0.0 0.03 (1)	7.81			
K-J	0 / 714	-28.0	-28.0 0.79 (2)	10.00			
J-I	0 / 714	-28.0	-28.0 0.79 (2)	10.00			
I-H	0 / 714	-28.0	-28.0 0.79 (2)	10.00			

TOTAL WEIGHT = 3 X 84 = 253 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 3.0 PSF
BOT CH. LL	= 10.5 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 46.1 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.62")
CALCULATED VERT. DEFL.(LL)= L/999 (0.21")
ALLOWABLE DEFL.(TL)= L/360 (0.62")
CALCULATED VERT. DEFL.(TL)= L/632 (0.35")

CSI: TC=0.30 (B-C:1), BC=0.79 (I-K:2), WB=0.94 (E-H:1), SSI=0.22 (I-K:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

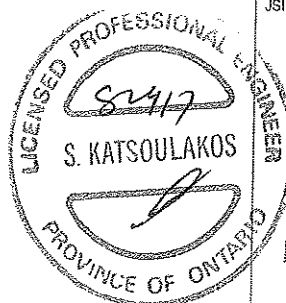
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

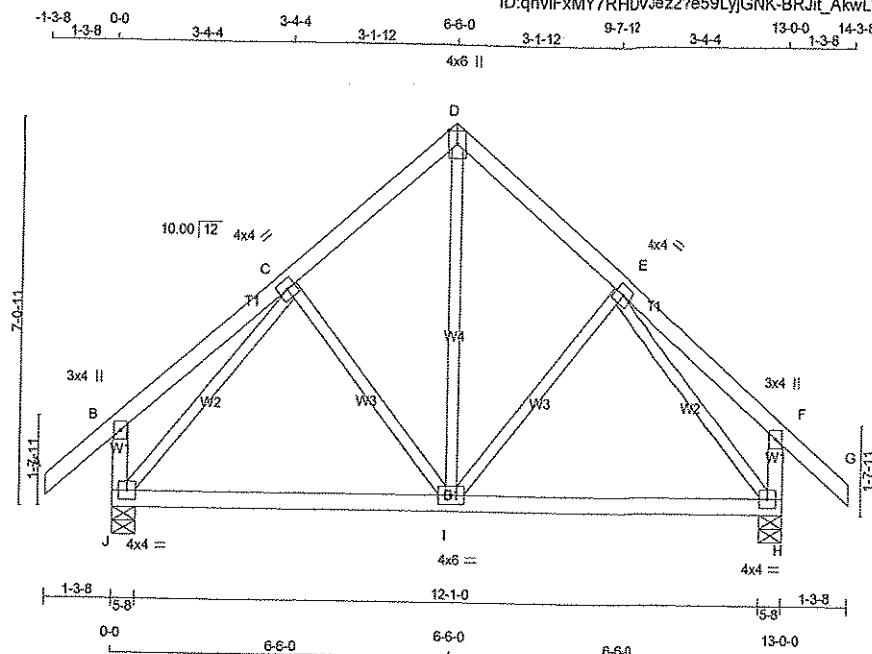
JSI GRIP= 0.88 (K) (INPUT = 0.90)
JSI METAL= 0.54 (J) (INPUT = 1.00)



DRWG NO. TAM 43572-17
STRUCTURAL
COMPONENT ONLY

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MITek Industries, Inc. Tue Aug 29 12:24:25 2017 Page 1
ID:qhviFxmY7RHDVJez2?e59LyjGNK-BRjIt_AkwLVAgT7BtM9S3VbhtZUsU?nroCic?2yj?b4



Scale = 1:42.5

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
J - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMVW1-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
J	846	0	846	0
H	846	0	846	0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	678	403 / 0	137 / 0	0 / 0	0 / 0	138 / 0	0 / 0
H	678	403 / 0	136 / 0	0 / 0	0 / 0	138 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)	UNBRACED LENGTH
FR-TO		FROM	TO	FR-TO			
A-B	0 / 37	-84.3	-84.3 0.12 (1)	I-D	0 / 407	0.09 (1)	10.00
B-C	0 / 20	-84.3	-84.3 0.14 (1)	I-E	-118 / 39	0.05 (1)	10.00
C-D	-506 / 0	-84.3	-84.3 0.11 (1)	C-I	-118 / 39	0.05 (1)	6.25
D-E	-506 / 0	-84.3	-84.3 0.11 (1)	J-C	-714 / 0	0.30 (1)	6.25
E-F	0 / 20	-84.3	-84.3 0.14 (1)	E-H	-714 / 0	0.30 (1)	10.00
F-G	0 / 37	-84.3	-84.3 0.12 (1)				
J-B	-221 / 0	0.0	0.0 0.02 (1)				7.81
H-F	-221 / 0	0.0	0.0 0.02 (1)				7.81
J-I	0 / 449	-28.0	-28.0 0.39 (2)				10.00
I-H	0 / 449	-28.0	-28.0 0.39 (2)				10.00

TOTAL WEIGHT = 2 X 62 = 124 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 3.0 PSF
BOT CH. LL	= 10.5 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.43")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.05")
ALLOWABLE DEFL.(TL) = $L/360$ (0.43")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.09")

CSI: TC=0.14 (B-C:1), BC=0.39 (I-J:2), WB=0.30 (C-J:1), SSI=0.15 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

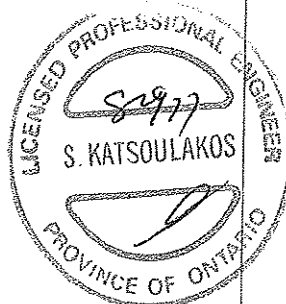
NAIL VALUES

PLATE GRIP(DRY) SHEAR (PSI)	SECTION (PLI)	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1657	822 2284 1656

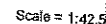
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (C) (INPUT = 0.90)
JSI METAL= 0.26 (C) (INPUT = 1.00)



DWG NO. TAM43571-17
STRUCTURAL
COMPONENT ONLY

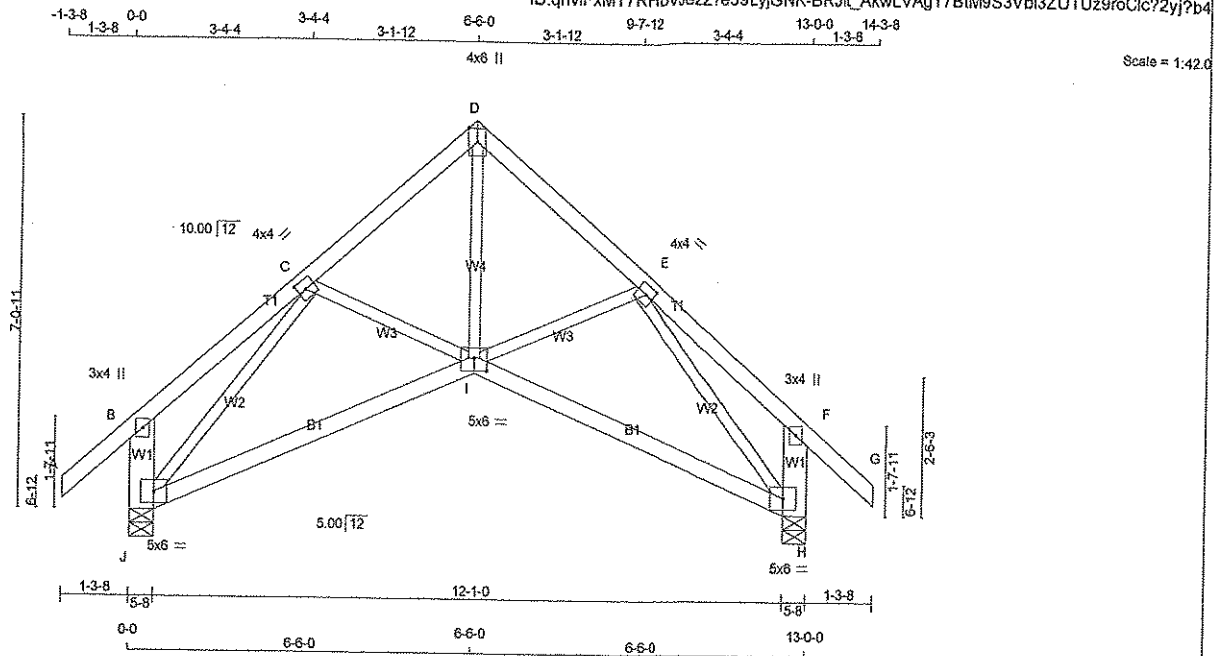


A circular professional seal for a Licensed Professional Engineer in the Province of Ontario. The seal contains the text "LICENSED PROFESSIONAL ENGINEER" around the top and "PROVINCE OF ONTARIO" around the bottom. In the center, the license number "82417" is written above the name "S. KATSOULAKOS". A stylized signature is written over the name.

WING NO. TAM 43575-17
STRUCTURAL
COMPONENT ONLY

Tamarack Roof Truss, Burlington

ID: qhviFxmY7RHdvJez2?e59LyjGNK-BRjIt_AkwLVAgT7BIM9S3Vbi3ZUTUz9roCic?2yj?b4



TOTAL WEIGHT = 2 X 81 = 122 lb [M/F]

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
J - B	2x6	DRY	No.2
H - F	2x6	DRY	No.2
J - I	2x4	DRY	No.2
I - H	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	4.0	2.00	1.75
F	TMV+p	MT20	3.0	4.0		
H	BVMW1-p	MT20	5.0	6.0		
I	BBWWW-p	MT20	5.0	6.0	2.75	3.00
J	BVMW1-p	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG IN-SX	REQD BRG IN-SX
JT	VERT	HORZ	DOWN	HORZ
J	846	0	846	0
H	846	0	846	0

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
J	678	403 / 0	136 / 0	0 / 0	0 / 0	138 / 0	0 / 0
H	678	403 / 0	137 / 0	0 / 0	0 / 0	138 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC) UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC) UNBRAC
FR-TO				FR-TO			
A-B	0 / 37	-84.3	-84.3 0.12 (1)	10.00	I-D	0 / 725	0.16 (1)
B-C	0 / 18	-84.3	-84.3 0.13 (1)	10.00	I-E	-45 / 66	0.01 (3)
C-D	-754 / 0	-84.3	-84.3 0.10 (1)	6.25	C-I	-45 / 66	0.01 (3)
D-E	-754 / 0	-84.3	-84.3 0.10 (1)	6.25	J-C	-990 / 0	0.41 (1)
E-F	0 / 18	-84.3	-84.3 0.13 (1)	10.00	E-H	-990 / 0	0.41 (1)
F-G	0 / 37	-84.3	-84.3 0.12 (1)	10.00			
J-B	-225 / 0	0.0	0.0 0.02 (1)	7.81			
H-F	-225 / 0	0.0	0.0 0.02 (1)	7.81			
J-I	0 / 657	-28.0	-28.0 0.42 (2)	10.00			
I-H	0 / 657	-28.0	-28.0 0.42 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 3.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL)= L/989 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL)= L/699 (0.22")

CSI: TC=0.13 (B-C:1), BC=0.42 (H-I:2), WB=0.41 (C-J:1), SSI=0.11 (H-I:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

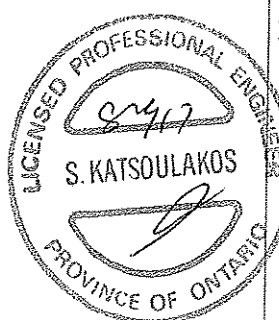
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	618	354	1667
	822	2284	1656

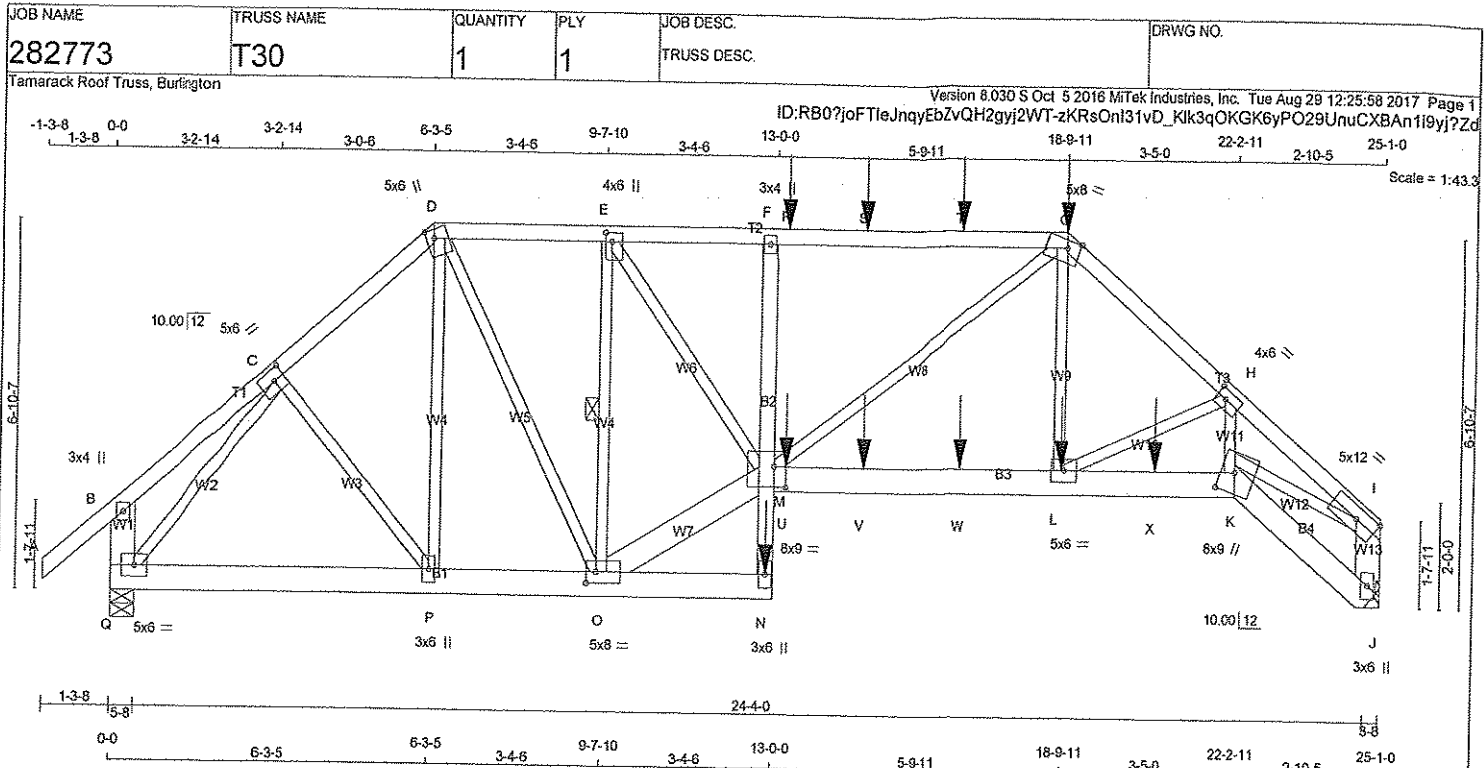
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (C) (INPUT = 0.90)
JSI METAL= 0.36 (C) (INPUT = 1.00)



DRWG NO. YAM 43572-17
STRUCTURAL
COMPONENT ONLY



LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
J - I	2x6	DRY	No.2	SPF
Q - B	2x6	DRY	No.2	SPF
Q - N	2x6	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
K - J	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
O - M	2x6	DRY	No.2	SPF
DRY, SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
C	TMWW-t	MT20	5.0	6.0	2.50 2.50
D	TTWW+m	MT20	5.0	6.0	2.00 1.75
E	TMWW-t	MT20	4.0	6.0	2.00 1.50
F	TMV+p	MT20	3.0	4.0	
G	TTWW-m	MT20	5.0	6.0	Edge 3.00
H	TMWW-t	MT20	4.0	6.0	2.00 2.25
I	TMV-t	MT20	5.0	12.0	Edge
J	BMV1+p	MT20	3.0	6.0	
K	BBWW+m	MT20	8.0	9.0	4.50 3.25
L	BMWW-t	MT20	5.0	6.0	
M	BVMWWW-t	MT20	8.0	9.0	4.50 2.75
N	BMV+p	MT20	3.0	6.0	
O	BMWWW-t	MT20	5.0	6.0	2.50 2.25
P	BMWWH	MT20	3.0	6.0	
Q	BMWW1-t	MT20	5.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 264.3 lbs FACTORED DOWN AT 18-9-11, 38.3 lbs FACTORED DOWN AT 13-2-12, AND 38.3 lbs FACTORED DOWN AT 14-8-15, AND 38.3 lbs FACTORED DOWN AT 16-8-15 ON TOP CHORD, AND 540.5 lbs FACTORED DOWN AT 12-10-4, 49.4 lbs FACTORED DOWN AT 13-2-12, 34.1 lbs FACTORED DOWN AT 14-8-15, 34.1 lbs FACTORED DOWN AT 16-8-15, AND 302.6 lbs FACTORED DOWN AT 20-7-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REORD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	BRG	IN-SX	BRG	IN-SX
J	2254	0	0	2254	0	0	HANGER BY OTHERS	0	0
Q	1987	0	0	1987	0	0	MIN. SEAT SIZE: 3-8	5-8	5-8

ALLOW FOR 0.3" OF HORIZONTAL MOVEMENT DUE TO TOTAL LOAD

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN COMPONENT REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
J	1815	1085 / 0	374 / 0	0 / 0	0 / 0	376 / 0	0 / 0		
Q	1604	935 / 0	335 / 0	0 / 0	0 / 0	334 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 137	-84.3 -84.3	0.13 (1)	C-P	0 / 190	0.05 (2)	
B-C	0 / 17	-84.3 -84.3	0.12 (1)	P-D	0 / 170	0.04 (3)	
C-D	-1958 / 0	-84.3 -84.3	0.18 (1)	D-O	0 / 1085	0.27 (1)	
D-E	-1985 / 0	-84.3 -84.3	0.22 (1)	O-E	-2075 / 0	0.63 (1)	
E-F	-3339 / 0	-84.3 -84.3	0.62 (1)	O-M	0 / 2302	0.30 (1)	
F-R	-3352 / 0	-84.3 -84.3	0.84 (1)	E-M	0 / 2326	0.58 (1)	
R-S	-3352 / 0	-84.3 -84.3	0.84 (1)	M-G	0 / 950	0.24 (1)	
S-T	-3352 / 0	-84.3 -84.3	0.84 (1)	L-G	0 / 923	0.23 (1)	
T-G	-3352 / 0	-84.3 -84.3	0.84 (1)	L-H	-1300 / 0	0.35 (1)	
G-H	-3350 / 0	-84.3 -84.3	0.35 (1)	K-H	0 / 1332	0.33 (1)	
H-I	-4960 / 0	-84.3 -84.3	0.50 (1)	K-I	0 / 3963	0.98 (1)	
I-J	-2214 / 0	0.0 0.0	0.17 (1)	Q-C	-2179 / 0	0.82 (1)	
Q-B	-222 / 0	0.0 0.0	0.02 (1)				
Q-P	0 / 1377	-28.0 -28.0	0.26 (1)				
P-O	0 / 1485	-28.0 -28.0	0.26 (2)				
O-N	0 / 30	-28.0 -28.0	0.05 (1)				
N-M	0 / 567	0.0 0.0	0.19 (1)				
M-F	-585 / 0	0.0 0.0	0.16 (1)				
M-U	0 / 2585	-28.0 -28.0	0.45 (1)				
U-V	0 / 2585	-28.0 -28.0	0.45 (1)				
V-W	0 / 2585	-28.0 -28.0	0.45 (1)				
W-L	0 / 2585	-28.0 -28.0	0.45 (1)				
L-X	0 / 3728	-28.0 -28.0	0.66 (1)				
X-K	0 / 3728	-28.0 -28.0	0.66 (1)				
K-J	0 / 0	-28.0 -28.0	0.04 (2)				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE
G	18-9-11	-20	-22	FRONT	VERT	DEAD
G	18-9-11	-42	-42	FRONT	VERT	TOTAL
G	18-9-11	-203	-203	FRONT	VERT	SNOW

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. O.C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.84")
CALCULATED VERT. DEFL.(LL) = L/999 (0.18")
ALLOWABLE DEFL.(TL) = L/360 (0.84")
CALCULATED VERT. DEFL.(TL) = L/999 (0.29")
CSI: TC=0.84 (F-G:1), BC=0.66 (K-L:1), WB=0.98 (K-K:1), SSI=0.32 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

AVAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (Q) (INPUT = 0.90)
JSI METAL= 0.93 (K) (INPUT = 1.00)

DRWG NO. TAM 43576-17

STRUCTURAL

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
282773	T30	1	1	TRUSS DESC.	
Tamarack Roof Truss, Burlington					

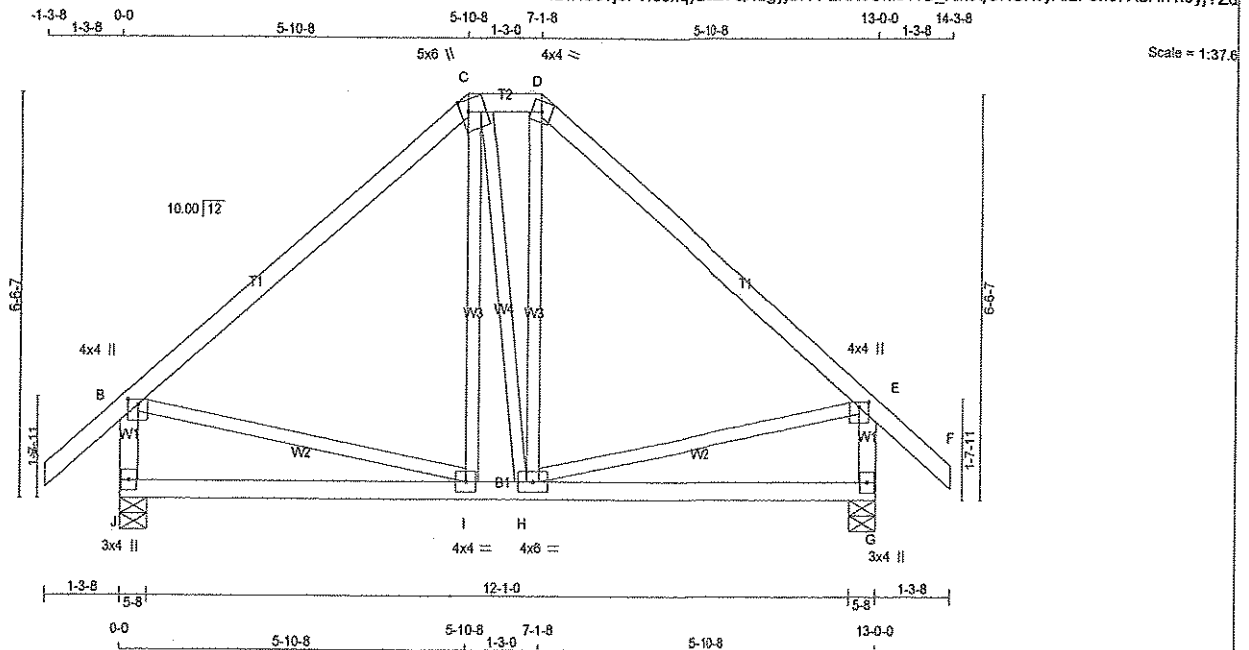
Version 8.030 S Oct 5 2016 MITek Industries, Inc. Tue Aug 29 12:25:58 2017 Page 2
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FACTORED CONCENTRATED LOADS (LBS)

JT	LOC	LC1	MAX-	MAX+	FACE	DIR.	TYPE
L	18-8-15	-20	-34	—	FRONT	VERT	TOTAL
N	12-10-4	-540	-540	—	FRONT	VERT	TOTAL
R	13-2-12	-38	-38	—	FRONT	VERT	TOTAL
S	14-8-15	-38	-38	—	FRONT	VERT	TOTAL
T	16-8-15	-38	-38	—	FRONT	VERT	TOTAL
U	13-2-12	-28	-49	—	FRONT	VERT	TOTAL
V	14-8-15	-20	-34	—	FRONT	VERT	TOTAL
W	16-8-15	-20	-34	—	FRONT	VERT	TOTAL
X	20-7-8	-303	-303	—	FRONT	VERT	TOTAL



NO. TAM 43576-17
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 65 lb [MIP]

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	DESCR.
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
J - B	2x4	DRY	No.2
G - E	2x4	DRY	No.2
J - G	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	4.0	4.0	1.00 2.00
C	TTWW+m	MT20	5.0	6.0	2.25 1.50
D	TTW-m	MT20	4.0	4.0	
E	TMVW+p	MT20	4.0	4.0	1.00 2.00
G	BMV1+p	MT20	3.0	4.0	
H	BMWW-t	MT20	4.0	6.0	
I	BMWW-t	MT20	4.0	4.0	
J	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	DOWN	BRG	BRG
J	846 0	846 0	0 0	5-8	5-8
G	846 0	846 0	0 0	5-8	5-8

UNFACTORED REACTIONS		1ST LCASE	MAX/MIN. COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
J	678	403 / 0	137 / 0	0 / 0	0 / 0	138 / 0	0 / 0
G	678	403 / 0	137 / 0	0 / 0	0 / 0	138 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

CHORDS		MAX. FACTORED	FACTORED	VERT. LOAD LC1	MAX	MAX	MEMB.	MAX. FACTORED	FORCE	MAX
FR-TO	MEMB.	FORCE (LBS)	FROM	TO	CSI (LC)	UNBRAC	LENGTH FR-TO	MEMB.	FORCE (LBS)	CSI (LC)
A-B	0 / 37	-84.3	-84.3	0.12 (1)	10.00	I-C	0 / 135	0.03 (3)		
B-C	-512 / 0	-84.3	-84.3	0.38 (1)	6.25	C-H	0 / 19	0.00 (3)		
C-D	-394 / 0	-84.3	-84.3	0.02 (1)	6.25	H-D	0 / 154	0.03 (3)		
D-E	-513 / 0	-84.3	-84.3	0.38 (1)	6.25	B-I	0 / 402	0.09 (1)		
E-F	0 / 37	-84.3	-84.3	0.12 (1)	10.00	H-E	0 / 402	0.09 (1)		
J-B	-779 / 0	0.0	0.0	0.08 (1)	7.81					
G-E	-778 / 0	0.0	0.0	0.08 (1)	7.81					
J-I	0 / 0	-28.0	-28.0	0.23 (3)	10.00					
I-H	0 / 393	-28.0	-28.0	0.30 (2)	10.00					
H-G	0 / 0	-28.0	-28.0	0.25 (3)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/999 (0.08")

CSI: TC=0.38 (D-E:1), BC=0.30 (H-I:2), WB=0.09 (E-H:1), SSI=0.15 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 L.S BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

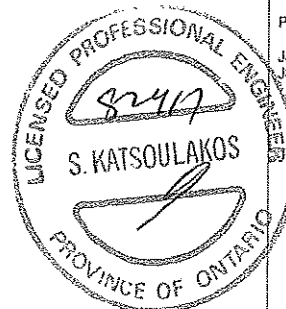
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1658

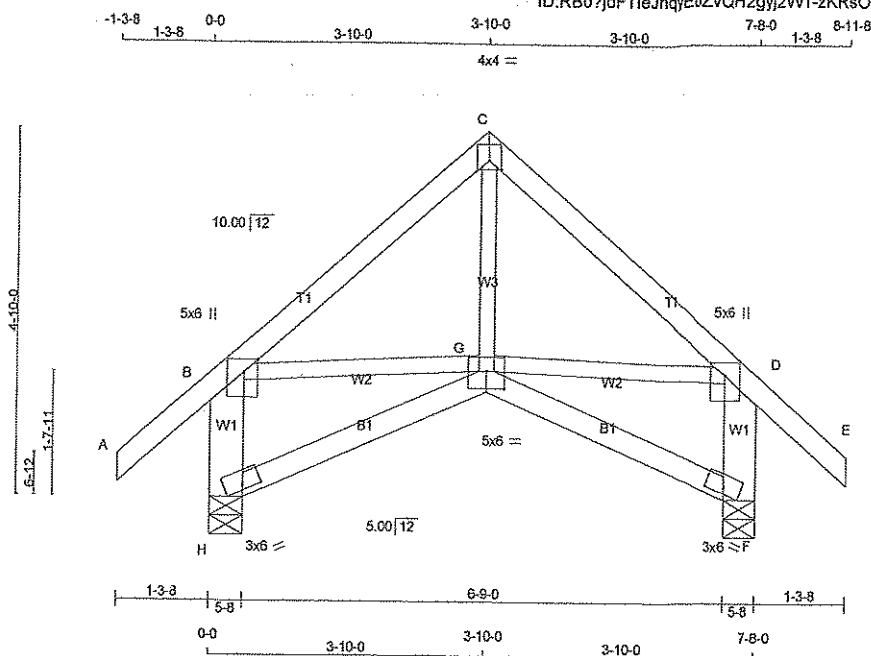
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.72 (E) (INPUT = 0.90)
JSI METAL= 0.20 (E) (INPUT = 1.00)



DRWG NO. TAM 43577-17
STRUCTURAL
COMPONENT ONLY



Scale = 1:30 g

TOTAL WEIGHT = $3 \times 38 = 114$ lb

LUMBER

N. L. G. A. RULES				
CHORDS	SIZE		LUMBER	DESCR
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x6	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY
EXCEPT

DRY: SEASONED LUMBER

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TTW-p	MT20	4.0	4.0	1.50	2.00
D	TMVW+p	MT20	5.0	6.0	2.00	2.25
F	BVM1-I	MT20	3.0	6.0	0.75	3.00
G	BBVWW-p	MT20	5.0	6.0	2.75	3.00
H	BVM1-I	MT20	3.0	6.0	0.75	3.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	547	0	547	0	0	5-8	5-8
F	547	0	547	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	432	267 / 0	81 / 0	0 / 0	0 / 0	85 / 0	0 / 0
F	432	267 / 0	81 / 0	0 / 0	0 / 0	85 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H. F.

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	FACTORED FORCE (LBS)	MAX CSI (LC)		
FR-TO		FROM TO			FR-TO				
A-B	0/37	-84.3	-84.3	0.12 (1)	10.00	G-C	0/199	0.04 (2)	
B-C	-348/0	-84.3	-84.3	0.16 (1)	6.25	B-G	0/268	0.06 (1)	
C-D	-348/0	-84.3	-84.3	0.16 (1)	6.25	G-D	0/268	0.08 (1)	
D-E	0/37	-84.3	-84.3	0.12 (1)	10.00				
H-B	-493/0	0.0	0.0	0.04 (1)	7.81				
F-D	-493/0	0.0	0.0	0.04 (1)	7.81				
H-G	0/0	-28.0	-28.0	0.12 (3)	10.00				
G-F	0/0	-28.0	-28.0	0.12 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.26")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.26")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.16 (C-D:1), BC=0.12 (F-G:3), WB=0.06 (B-G:1), SSI=0.10 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

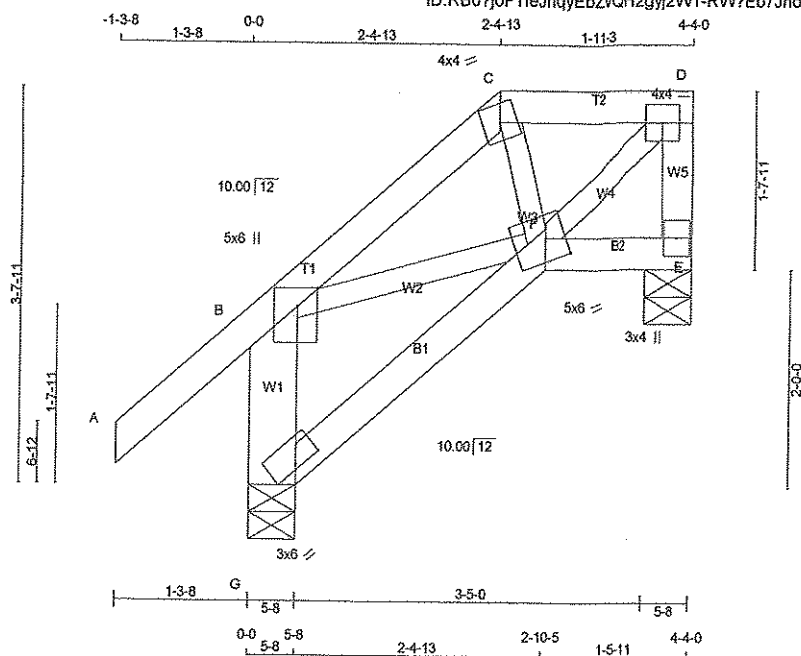
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.66 (C) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)



DWG NO. TAM 4357B 17
STRUCTURAL
COMPONENT ONLY



Scale = 1:21.5

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	DRY	LUMBER
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
E - D	2x4	DRY	No.2
G - B	2x6	DRY	No.2
G - F	2x4	DRY	No.2
F - E	2x4	DRY	No.2

ALL WEBS	EXCEPT	2x3	DRY	No.2	SPF
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DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2'-0" O.C.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TTW-m	MT20	4.0	4.0		
D	TMVW-l	MT20	4.0	4.0		
E	BMV1+p	MT20	3.0	4.0		
F	BBWWV-m	MT20	5.0	6.0	2.50	2.25
G	BVM1-l	MT20	3.0	6.0	0.50	3.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	JT	GROSS REACTION	JT	BRG	JT	BRG
E	243	0	243	0	5-8	5-8	5-8
G	380	0	380	0	5-8	5-8	5-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	200	111 / 0	45 / 0	0 / 0	0 / 0	43 / 0	0 / 0
G	279	182 / 0	45 / 0	0 / 0	0 / 0	52 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 37	-84.3 -84.3	0.12 (1)	10.00	C-F	-41 / 47	0.01 (3)
B-C	-226 / 0	-84.3 -84.3	0.08 (1)	6.25	F-D	0 / 214	0.05 (1)
C-D	-159 / 0	-84.3 -84.3	0.05 (1)	6.25	B-F	0 / 178	0.04 (1)
E-D	-223 / 0	0.0 0.0	0.02 (1)	7.81			
G-B	-320 / 0	0.0 0.0	0.02 (1)	7.81			
G-F	0 / 0	-28.0 -28.0	0.07 (3)	10.00			
F-E	0 / 0	-28.0 -28.0	0.02 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/899 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/899 (0.01")

CSI: TC=0.12 (A-B:1), BC=0.07 (F-G:3), WB=0.05 (D-F:1), SS=0.07 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

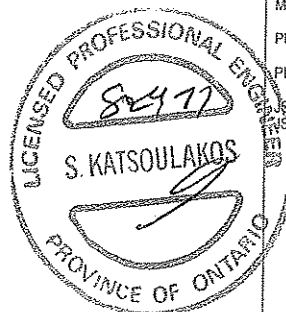
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

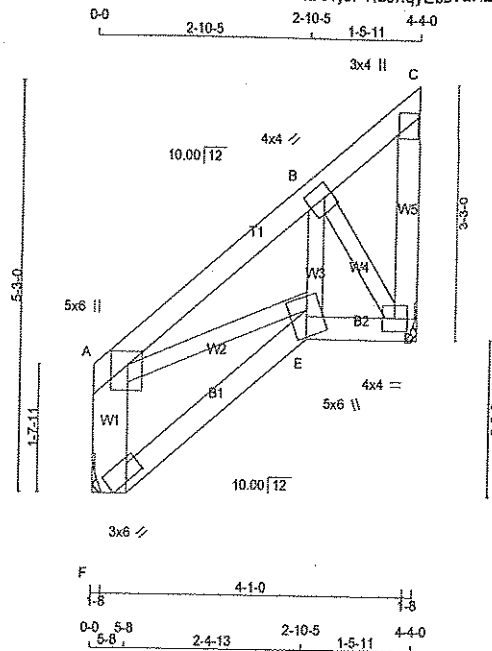
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.36 (F) (INPUT = 0.90)
SI METAL= 0.08 (B) (INPUT = 1.00)



DWG NO. TAM 43579-17
STRUCTURAL
COMPONENT ONLY



Scale = 1:29.6

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x6	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	2.00	2.25
B	TMVW-l	MT20	4.0	4.0	2.00	1.25
C	TMV+p	MT20	3.0	4.0		
D	BMVW1-l	MT20	4.0	4.0		
E	BBWW+m	MT20	5.0	6.0		
F	BVM1-l	MT20	3.0	6.0	0.50	3.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REORD BRG
	VERT	HORZ	DOWN	HORZ		
D	243	0	243	0	0	
F	243	0	243	0	0	

HANGER BY OTHERS MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	MAX/MIN COMPONENT REACTIONS				
		SNOW	LIVE	PERM. LIVE	WIND	DEAD
D	200	111 / 0	45 / 0	0 / 0	0 / 0	43 / 0
F	200	111 / 0	45 / 0	0 / 0	0 / 0	43 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)		FACTORED VERT. LOAD LC1 (PLF)		MAX. UNBRACED LENGTH	MEMB.	WEBS MAX. FACTORED FORCE (LBS)	
	FR-TO	FROM	TO	TO			FR-TO	FROM
A-B	-209 / 0	-84.3	-84.3	0.08 (1)	6.25	A-E	0 / 176	0.04 (1)
B-C	-29 / 0	-84.3	-84.3	0.07 (1)	6.25	E-B	0 / 137	0.03 (2)
D-C	-12 / 0	0.0	0.0	0.00 (1)	7.81	B-D	-269 / 0	0.04 (1)
F-A	-203 / 0	0.0	0.0	0.02 (1)	7.81			
F-E	0 / 0	-28.0	-28.0	0.07 (2)	10.00			
E-D	0 / 164	-28.0	-28.0	0.04 (1)	10.00			

TOTAL WEIGHT = 23 lb

[M][P]

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL)= L/999 (0.01")

CSI: TC=0.08 (A-B:1), BC=0.07 (E-F:2), WB=0.04 (B-D:1), SSI=0.09 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

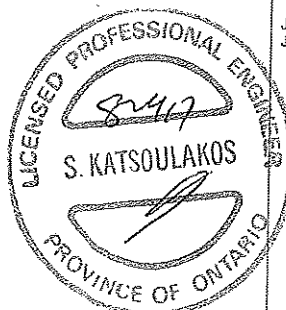
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (D) (INPUT = 0.90)
JSI METAL= 0.06 (D) (INPUT = 1.00)



DRWG NO. TAM 43580-17
STRUCTURAL
COMPONENT ONLY

LUS - Double Shear Joist Hangers



All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

MATERIAL: 18 gauge

FINISH: G90 galvanized

DESIGN:

- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

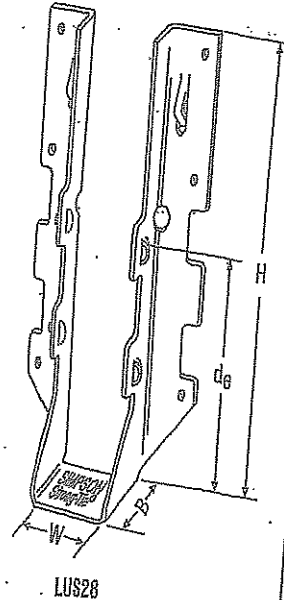
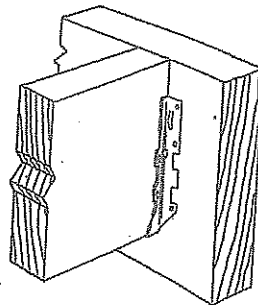
INSTALLATION:

- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

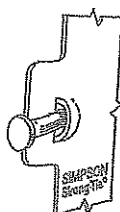
- These hangers cannot be modified.

Typical LUS Installation



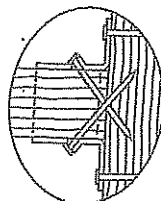
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
		W	H	B	d _g ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _u =1.15)	Normal (K _p =1.00)	Uplift (K _u =1.15)	Normal (K _p =1.00)
LUS24	18	1 1/8	3 1/4	1 1/4	1 1/8	4-10d	2-16d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/4	2	1 1/8	4-16d	2-16d	835	2020	590	1435
LUS26	18	1 1/8	4 1/4	1 1/4	3/8	4-10d	4-10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 1/4	2	4	4-16d	4-16d	1720	2595	1545	1920
LUS26-3	18	4 1/8	4 1/4	2	3/4	4-16d	4-16d	1720	2595	1545	1920
LUS28	18	1 1/8	6 1/4	1 1/4	3/4	6-16d	4-10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	6-16d	4-16d	1720	3325	1545	2575
LUS28-3	18	4 1/8	6 1/4	2	3/4	6-16d	4-16d	1720	3325	1545	2375
LUS210	18	1 1/8	7 3/4	1 1/4	3/8	8-16d	4-10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	8-16d	6-16d	2580	4500	2320	3195
LUS210-3	18	4 1/8	8 1/4	2	5 1/4	8-16d	6-16d	2580	3345	2320	2375

1. d_g is the distance from the seat of the hanger to the highest joist nail.

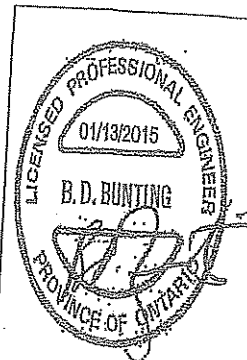


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Top View.



This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2016. This information is updated periodically and should not be relied upon after December 31, 2016. Contact Simpson Strong-Tie for current information and limited warranty or see www.strongtie.com.

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1. SPEC1615 1/15 exp. 12/16

800-999-5039

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HUS/LJS - Double Shear Joist Hangers



All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: See table

FINISH: G90 galvanized

DESIGN:

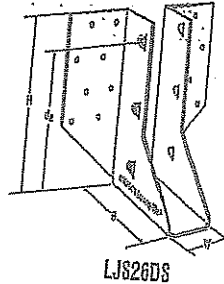
- Factored resistances are in accordance with CSA 086-14
- Uplift resistances have been increased 15%. No further increase is permitted
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

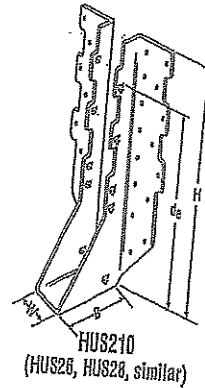
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

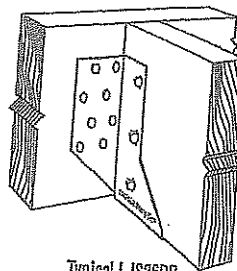
- See current catalogue for options



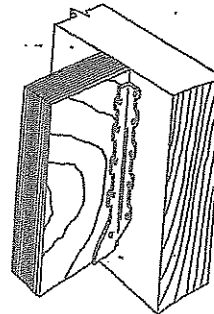
LJS26DS



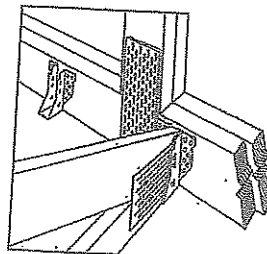
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS Installation



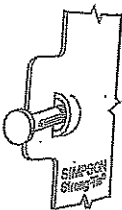
Typical HUS Installation



Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

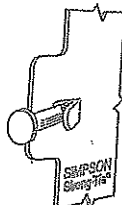
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
		W	H	B	d _g ¹	Face	Joist	D-Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
LJS26DS	18	1 1/16	5	3 1/2	4 3/4	16-16d	6-16d	2065	4265	1460	4115
HUS26	16	1 1/8	5 1/8	3	3 1/8	14-16d	6-16d	2705	4940	2065	3875
HUS28	16	1 1/8	7 3/8	3	6 1/2	22-16d	8-16d	3605	5365	2875	4345
HUS210	16	1 1/8	9 3/8	3	7 3/8	30-16d	10-16d	4505	5795	4010	4740
HUS1.61/10	16	1 1/8	9	3	8	30-16d	10-16d	4605	6450	4010	5200

1. d_g is the distance from the seat of the hanger to the highest joist nail.

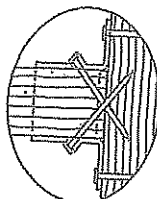


Dorne Double Shear Nailing prevents tabs breaking off (available on some models).

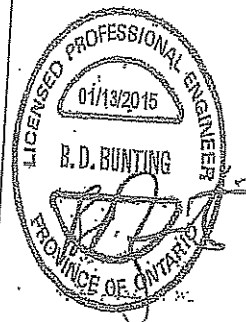
U.S. Patent
5,603,560



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



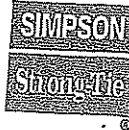
UL Classified Design...
2015...
2015...
2015...

800-999-5099

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SPCHUS16-1/16-12/16

HGUS - Double Shear Joist Hangers



All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: 12 gauge

FINISH: G90 galvanized

DESIGN:

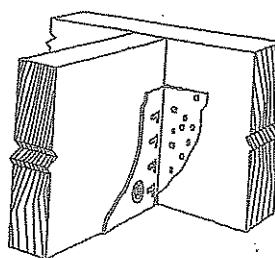
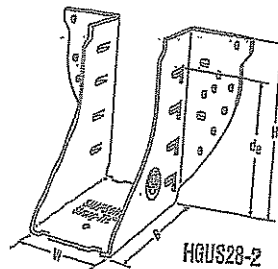
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%.
No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

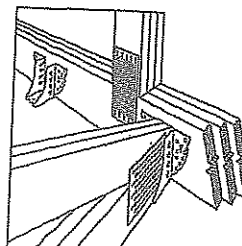
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

- See current catalogue for options.



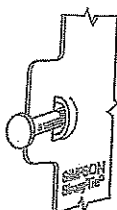
Typical HGUS Installation



Typical HGUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

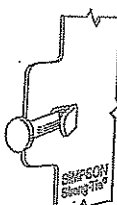
Model No.	Ga	Dimensions (in)					Fasteners		Factored Resistances (lbs)			
		W	H	B	d ₁	d ₂	Face	Joist	D.Fir-L		S-P-F	
									Uplift (K _u =1.15)	Normal (K _n =1.00)	Uplift (K _u =1.15)	Normal (K _n =1.00)
HGUS26	12	1 1/2	5 1/2	5	4 1/2	20-16d	8-16d	2685	6625	2685	5700	
HGUS28-2	12	3 1/2	5 1/2	4	4 1/2	20-16d	8-16d	4385	8950	3100	6355	
HGUS28-3	12	4 1/2	5 1/2	4	4 1/2	20-16d	8-16d	4385	8950	3100	6355	
HGUS28-4	12	6 1/2	5 1/2	4	4 1/2	20-16d	8-16d	4385	8950	3100	6355	
HGUS28	12	1 1/2	7 1/2	5	6 1/2	36-16d	12-16d	3310	7675	3100	6900	
HGUS28-2	12	3 1/2	7 1/2	4	6 1/2	36-16d	12-16d	6070	12980	4310	9215	
HGUS28-3	12	4 1/2	7 1/2	4	6 1/2	36-16d	12-16d	6070	12980	4310	9215	
HGUS28-4	12	6 1/2	7 1/2	4	6 1/2	36-16d	12-16d	6070	12980	4310	9215	
HGUS210-2	12	3 1/2	9 1/2	4	8 1/2	46-16d	16-16d	6840	14645	4855	10400	
HGUS210-3	12	4 1/2	9 1/2	4	8 1/2	46-16d	16-16d	6840	14645	4855	10400	
HGUS210-4	12	6 1/2	9 1/2	4	8 1/2	46-16d	16-16d	6840	14645	4855	10400	
HGUS212-4	12	6 1/2	10 1/2	4	10 1/2	58-16d	20-16d	7640	14995	5425	10645	
HGUS214-4	12	6 1/2	12 1/2	4	11 1/2	68-16d	22-16d	10130	16400	7195	11845	

1. d₂ is the distance from the seat of the hanger to the highest joist nail.

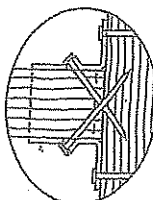


Double Double Shear Nailing prevents tabs breaking off (available on some models).

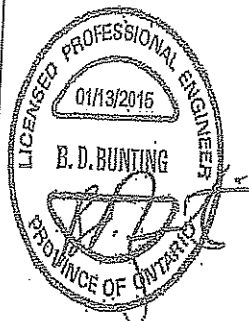
U.S. Patent
5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



DESIGN

Technical Bulletin 15-1715, dated 12/15/15, and its contents are for informational purposes only. This information is provided as a guide and should not be relied upon for design or construction. © 2015 Simpson Strong-Tie Company Inc.

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15-SPEC-HGUS-15-1715-10-12/15

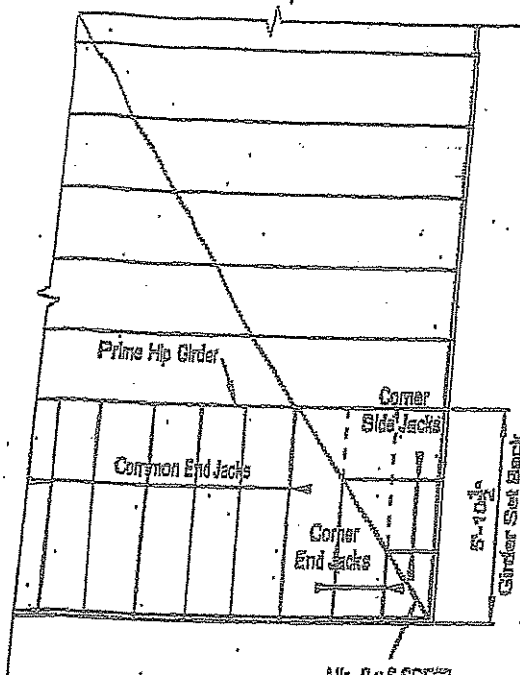
800-999-5099

www.strongtie.com

MICRO CITY

ENGINEERING SERVICES INC.

TEL: (519) 287-2242



45° Hip End

LUMBER SPECIFICATION

TOP CHORD : 2x4 SPF#2

BOTTOM CHORD : 2x4 SPF#2

WEBS : 2x3 SPF#2

UNLESS OTHERWISE SHOWN

DESIGN LOAD:

TOP CHORD LIVE LOAD : 39.4 P.S.F.

TOP CHORD DEAD LOAD : 3.0 P.S.F.

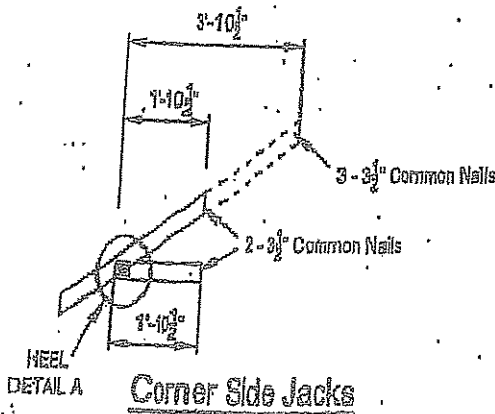
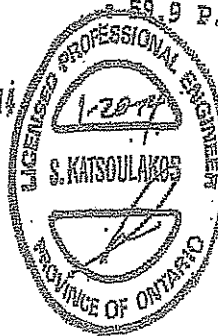
BOTTOM CHORD LIVE LOAD : 10.5 P.S.F.

BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

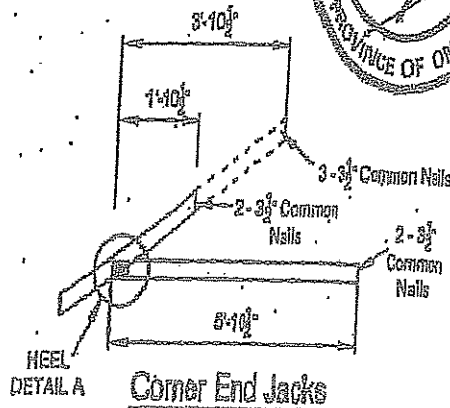
TOTAL LOAD : 59.9 P.S.F.

DWG NO. TIM 2495.11

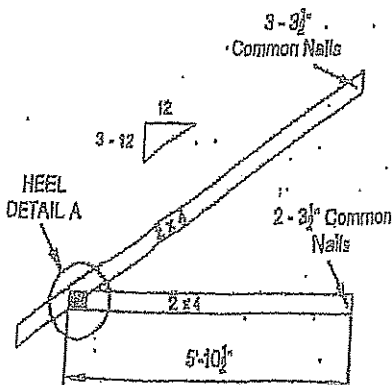
STRUCTURAL
COMPONENT ONLY



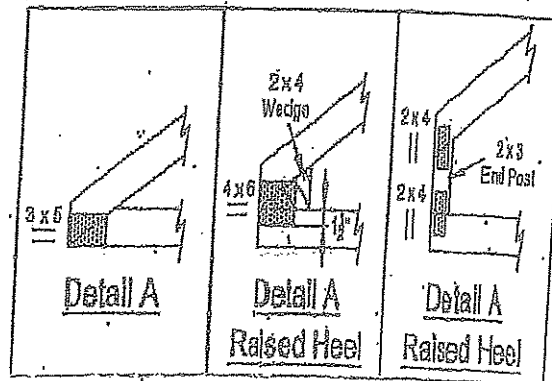
Corner Side Jacks



Corner End Jacks



Common End Jacks



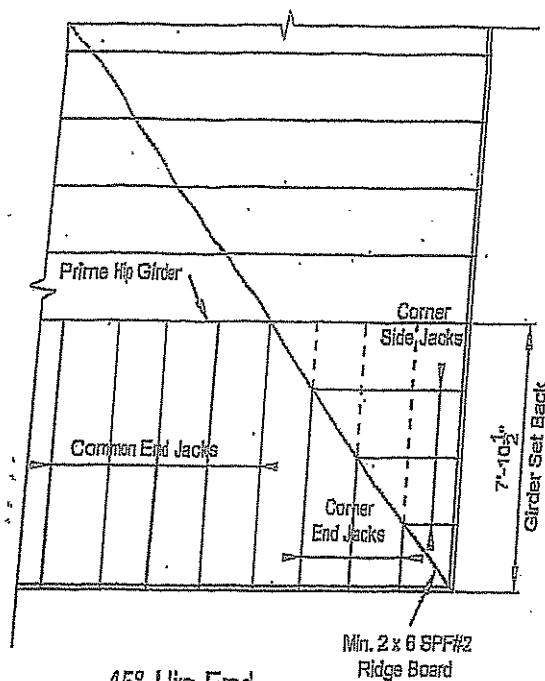
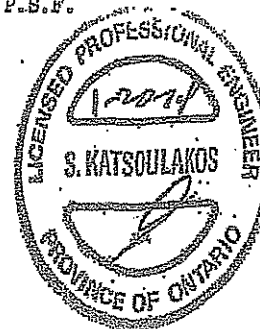
NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)

LUMBER SPECIFICATION

TOP CHORD : 2x4 SPF#2
 BOTTOM CHORD : 2x4 SPF#2
 WEBS : 2x3 SPF#2
 UNLESS OTHERWISE SHOWN

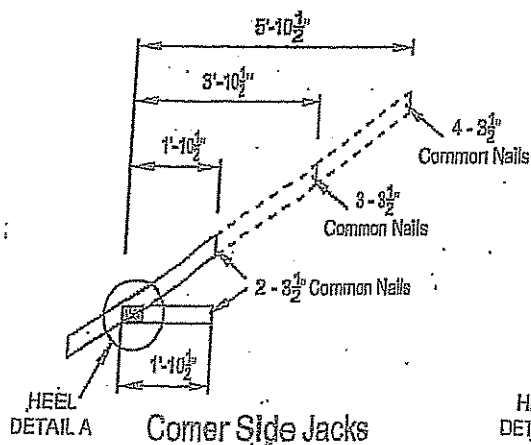
DESIGN LOAD:

TOP CHORD LIVE LOAD : 34.8 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.
 TOTAL LOAD : 44.8 P.S.F.

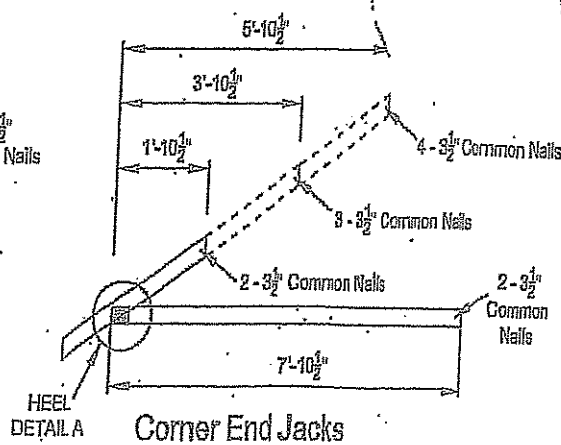


45° Hip End

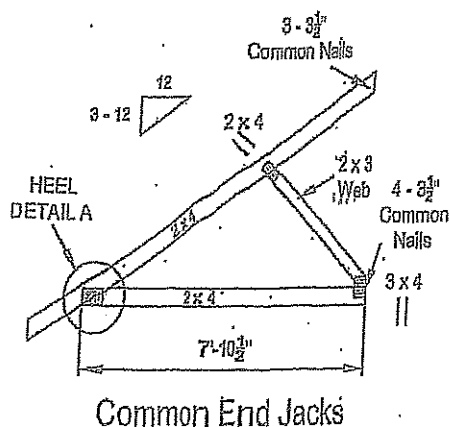
DWG NO. T1M 3503 T4
 STRUCTURAL
 COMPONENT ONLY



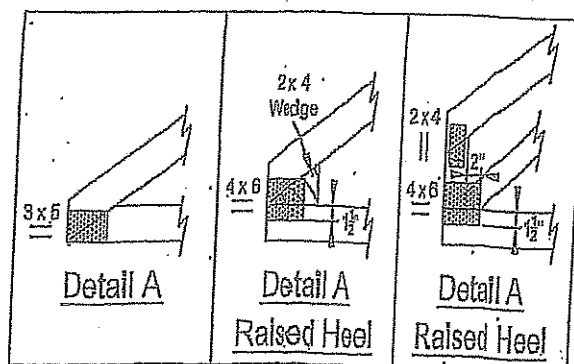
Corner Side Jacks



Corner End Jacks



Common End Jacks



Detail A

Detail A

Raised Heel

Detail A

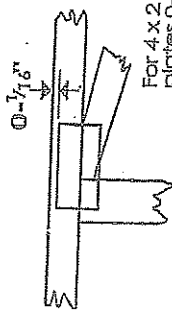
Raised Heel

NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
 (TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

Symbols

PLATE LOCATION AND ORIENTATION

Center plate on joint unless x, y offsets are indicated. Dimensions are in 1/16-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 1/4\" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

* Plate location details available in Mitek software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, L or Eliminator bracing if indicated.

BEARING



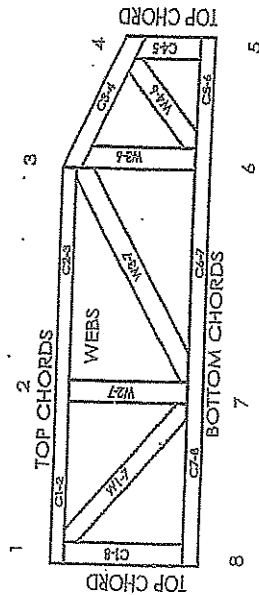
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

TPI-C: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in 1/16-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

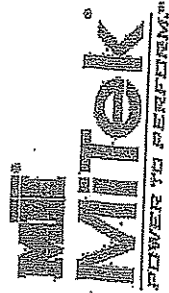
CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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Mitek Engineering Reference Sheet: MIII-7473C rev. 10-08

General Safety Notes

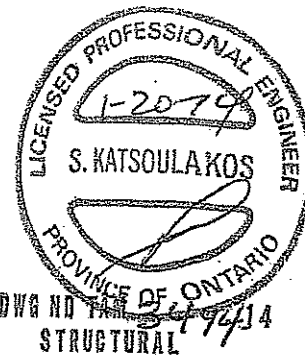
Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g., diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, L, or Eliminator bracing should be considered.
3. Never exceed the design loadings shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and waste at joint locations are regulated by TPI-C.
7. Design assumes trusses will be suitably protected from the environment in accord with TPI-C.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPI-C Quality Criteria.

Micro City Engineering Services Inc.
(BCIN: 26064; FIRM BCIN: 29991)

RR #1, Po Box 61
Glencoe, Ontario
N0L 1M0

(519) 287 - 2242; Fax: (519) 287 - 5750 (Call)



Responsibilities:

Micro City Engineering Services is responsible for the design of trusses as individual components.

It is the responsibilities of others to ascertain that the design loads utilized on this (these) drawing(s) meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdiction over such decisions.

All dimensions are to be verified by the owner, contractor, architect, or other authority having input over such decisions prior to truss component manufacture. At no time shall Micro City Engineering Services Inc. or its employees be responsible for dimension errors.

Micro City Engineering Services Inc. bears no responsibility for the erection of any truss components. Persons erecting truss components are cautioned to seek professional advice regarding temporary and permanent bracing systems and to be totally familiar with all aspects of truss erection prior to proceeding on any truss component erection job. Any bracing shown on Micro City Engineering Services Inc. or Tamarack Roof Trusses Inc. sealed or unsealed truss component drawings is specified for the single truss component in question and is identified as an integral part of the design for that particular truss component but is not meant to represent the only required bracing for that particular truss component when installed as a component in a series of truss components in a roof truss system.

~~It is the truss manufacturer's responsibility to ensure that trusses are manufactured in accordance with Micro City Engineering Services Inc. specifications outlined below:~~

SPECIFICATIONS:

Truss components sealed by Micro City Engineering Services Inc. must conform to the relevant sections of the current Building Code of Ontario and Canada (Part 4 or Part 9) or the current Farm Building Code of Canada in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the Truss Plate Institute of Canada (TPIC). All unit lumber and nailing stresses identified on truss component design drawings and/or used in the design of individual truss components shall conform to the current CSA Wood Design standard identified in the current Building Code and TPIC Design Standards.

The lumber used to manufacture any truss component is to conform to the specified size and grade identified on the truss drawing.

The lumber used in the manufacture of any truss component is not to exceed 19% during its service use unless specifically noted on the truss drawing.

The lumber used in the manufacture of any truss component is not to be treated with any chemicals during its service life unless specifically noted on the truss drawing.

Connector plates shall be applied to both faces of the truss component at each joint and shall be positioned exactly as specified.

The top chord of any truss component is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the sealed truss component drawing but not exceeding 24" o/c (Part 9 design) and not exceeding 48" o/c (Part 4 or Agricultural design).

When a truss component is to be installed with no rigid ceiling attached directly to the bottom chord, then the bottom chord is to be laterally braced at intervals not exceeding 3m (or 10'-0").

All sealed or unsealed truss component drawings provided by Micro City Engineering Services Inc. Or Tamarack Roof Trusses Inc. should be read in conjunction with the following:

Warning-Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473C rev 10-'08 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer - not the truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component Safety Information available from the Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA, 22314.