



Job Track: **43954**
Layout ID: **284265**
Plan Log: **92966**

Builder / Location:
GREENPARK HOMES / WATERDOWN

Model / Elevation:
ROSEWOOD 3 / 1

Project: **RUSSEL GARDENS PHASE 2**
Date: **8/25/2017** Designer: **colin/s.v.e**

M10,978

T - 170440

ASPHALT SHINGLES
12" FINISHED O.H.
5" CLADDING ALLOWANCE
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD
HEEL: R.T.M.C.

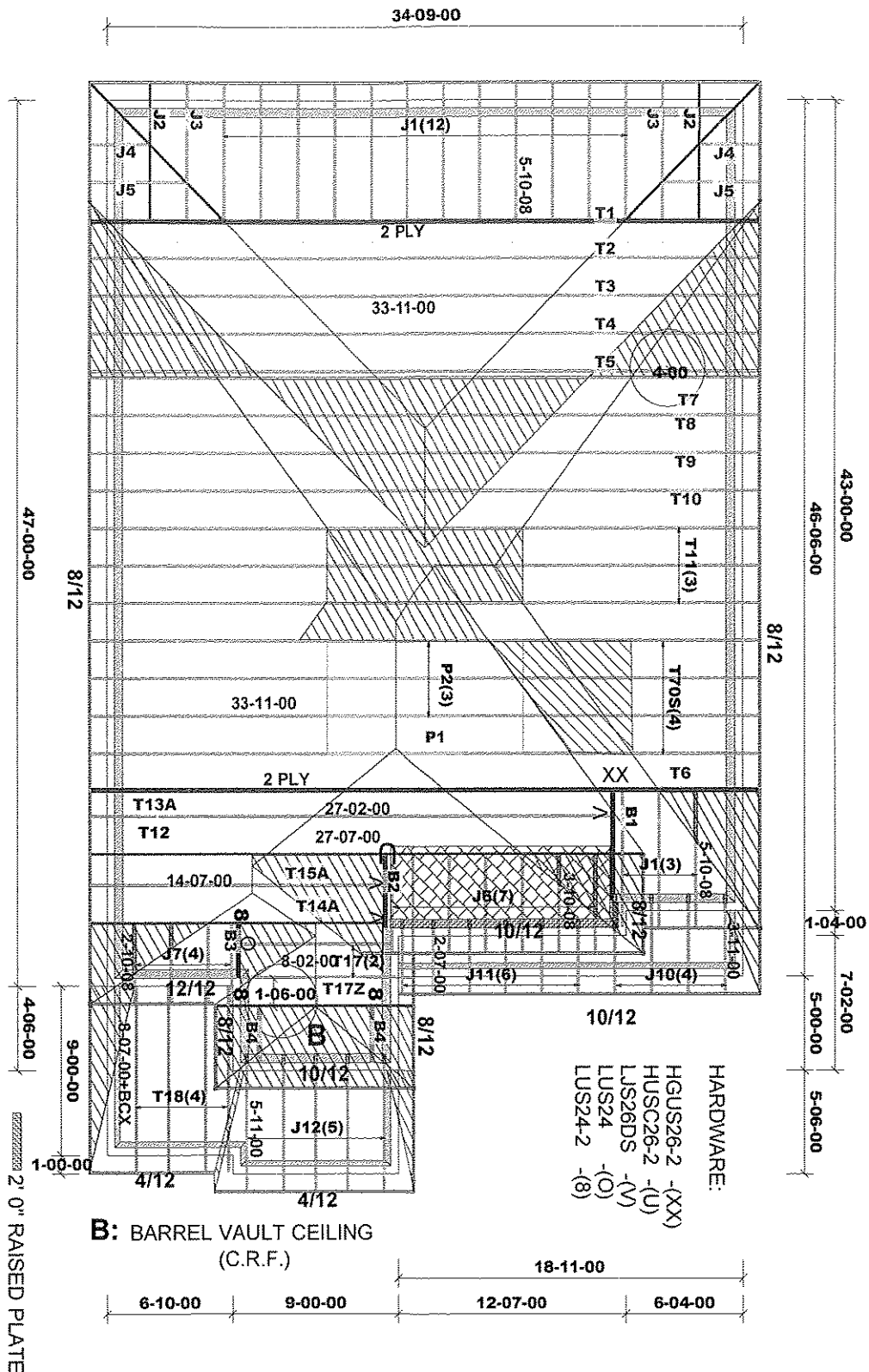
ALL CONV. FRAMING TO CONFORM
WITH PART 9 OF O.B.C. 2012 ROOF
RAFTERS THAT CROSS OVER OR
MEET TRUSSES TO BE 2X4 SPF #2 24"
O.C. WITH A VERT. POST TO THE TRUSS
UNDERNEATH AT EACH CROSS PT.
VERT. POST LONGER THAN 6'
TO HAVE LATERAL BRACING SO
THAT THE DIST. BETWEEN END PT.
& BETWEEN ROWS OF BRACING
DOES NOT EXCEED 6'

DESIGN CONFORMS WITH OBC 2012
OCCUPANCY: RESIDENTIAL | PART: 9
GROUND SNOW LOAD Ss=31.3psf S_t=8.4psf
DESIGN LOADS
TCOL (3psf)
BCLL (10.5psf)
BCDL (7psf)

ALL ROOF SLOPES ARE 6/12
UNLESS OTHERWISE NOTED

CONVENTIONAL
FRAMING

2' 0" RAISED CEILING
2' 0" RAISED PLATE



B: BARREL VAULT CEILING
(C.R.F.)

HARDWARE:
HUS26-2 -(XX)
HUSC26-2 -(U)
LUS26DS -(V)
LUS24 -(O)
LUS24-2 -(8)





Delivery Shiplist

DATE	08/25/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 284265 LOCATION: WATERDOWN
 BUILDER: GREENPARK - RUSSELL GARDENS PH2 SUB-BUILDER:
 MODEL: ROSEWOOD 3 ELEVATION: 1

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	T1 HIP GIRDER	6.00	33-11-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-02-00 01-02-00	333.02		
	2 Ply		0.00							208.68		
	1	T2 HIP	6.00 0.00	33-11-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	133.28 84.67		
	1	T3 HIP	6.00 0.00	33-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	135.85 85.67		
	1	T4 HIP	6.00 0.00	33-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	141.99 89.17		
	1	T5 HIP	6.00 0.00	33-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	143.06 91.00		
	1	T6 HIP GIRDER	8.00	33-11-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-04-13 01-04-13	361.72		
	2 Ply		0.00							224.00		
	4	T70S PIGGYBACK	8.00 0.00	33-11-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	623.64 403.32		
	1	T7 HIP	8.00 0.00	33-11-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	137.63 89.83		
	1	T8 HIP	8.00 0.00	33-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	141.70 91.50		
	1	T9 HIP	8.00 0.00	33-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	148.63 95.50		
	1	T10 HIP	8.00 0.00	33-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	149.48 94.33		
	3	T11 HIP	8.00 0.00	33-11-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	467.85 295.50		
	1	T12 HIP GIRDER	8.00	27-07-00	06-10-07	2 X 6	2 X 6	01-03-08 01-05-00	01-04-13 01-04-13	180.28		
			0.00							110.67		
	1	T13A HIP	8.00 0.00	27-02-00	08-06-07	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 03-08-02	124.79 78.67		
	1	T14A HIP GIRDER	8.00 0.00	14-07-00	04-09-00	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-08-02	59.57 38.17		
	1	T15A COMMON	8.00 0.00	14-07-00	06-04-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-08-02	61.52 38.33		
	2	T17 COMMON	8.00	08-02-00	04-01-08	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	71.40		
			0.00							45.34		
	1	T17Z COMMON	8.00	08-02-00	04-01-08	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	35.70		
			0.00							22.67		



Delivery Shiplist

DATE	08/25/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 284265 LOCATION: WATERDOWN
 BUILDER: GREENPARK - RUSSELL GARDENS PH1 SUB-BUILDER:
 MODEL: ROSEWOOD 3 ELEVATION: 1

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	4	T18 MONOPITCH	4.00 0.00	08-07-00	03-04-08	2 X 4	2 X 4	01-03-08 00-00-00	00-06-03 00-03-08	129.24 83.32		
	1	P1 PIGGYBACK	8.00 0.00	05-11-13	02-01-00	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 00-04-07	20.58 13.00		
	3	P2 PIGGYBACK	8.00 0.00	05-11-13	02-04-06	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 00-04-07	55.41 35.01		
	15	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	251.85 160.05		
	2	J2 JACK-OPEN	6.00 0.00	05-10-08	02-00-12	2 X 4	2 X 4	01-03-08 -04-01-01	01-02-00 00-03-08	32.32 20.66		
	2	J3 JACK-OPEN	6.00 0.00	05-10-08	03-00-12	2 X 4	2 X 4	01-03-08 -02-01-01	01-02-00 00-03-08	37.42 23.34		
	2	J4 JACK-OPEN	6.00 0.00	01-10-08	02-00-12	2 X 4	2 X 4	01-03-08 -00-01-01	01-02-00 00-03-08	16.86 11.34		
	2	J5 JACK-OPEN	6.00 0.00	01-10-08	03-00-12	2 X 4	2 X 4	01-03-08 01-10-15	01-02-00 00-04-11	21.96 14.00		
	7	J6 JACK-OPEN	10.00 0.00	03-10-08	04-10-07	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 04-10-07	96.25 60.69		
	4	J7 JACK-OPEN	12.00 0.00	02-10-08	04-09-00	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 04-09-00	48.00 32.00		
	4	J10 JACK-OPEN	10.00 0.00	03-11-00	03-09-14	2 X 4	2 X 4	01-03-08 00-00-00	00-06-11 03-09-14	51.48 34.68		
	6	J11 JACK-OPEN	10.00 0.00	02-07-00	02-08-08	2 X 4	2 X 4	01-03-08 00-00-00	00-06-11 02-08-08	56.22 40.02		
	5	J12 JACK-OPEN	4.00 0.00	05-11-00	02-05-14	2 X 4	2 X 4	01-03-08 00-00-00	00-06-03 00-05-06	82.60 50.00		

TOTAL # TRUSS= 83.00

TOTAL BFT OF ALL TRUSSES=

2765.13 BFT. TOTAL WEIGHT OF ALL TRUSSES= 4351.30 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	HGUS26-2	
1	Hangers	HUSC26-2	
1	Hangers	LUS24-2	
3	Hangers	LJS26DS	
1	Hangers	LUS24	

TOTAL # ITEMS= 7.00



Delivery Shiplist

DATE	08/25/17
SALES REP	Mario

JOB TRACK:43954 LAYOUT ID: 284266 LOCATION: WATERDOWN
 BUILDER: GREENPARK - RUSSELL GARDENS PH 1 SUB-BUILDER:
 MODEL: ROSEWOOD 3 ELEVATION: 2

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	T1 HIP GIRDER	6.00	33-11-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-02-00 01-02-00	333.02		
	2 Ply		0.00							208.68		
	1	T2 HIP	6.00 0.00	33-11-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	133.28 84.67		
	1	T3 HIP	6.00 0.00	33-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	135.85 85.67		
	1	T4 HIP	6.00 0.00	33-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	141.99 89.17		
	1	T5 HIP	6.00 0.00	33-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	143.06 91.00		
	4	T60S PIGGYBACK	8.00 0.00	33-11-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	599.24 385.32		
	1	T6Z HIP GIRDER	8.00	33-11-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-04-13 01-04-13	361.72		
	2 Ply		0.00							224.00		
	1	T7 HIP	8.00 0.00	33-11-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	137.63 89.83		
	1	T8 HIP	8.00 0.00	33-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	141.70 91.50		
	1	T9 HIP	8.00 0.00	33-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	148.63 95.50		
	1	T10 HIP	8.00 0.00	33-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	149.48 94.33		
	3	T11 HIP	8.00 0.00	33-11-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	467.85 295.50		
	1	T20 HIP GIRDER	8.00	27-07-00	06-10-07	2 X 6	2 X 6	01-03-08 00-00-00	01-04-13 01-04-13	173.21		
			8.00							107.67		
	1	T21 HIP	8.00 0.00	27-07-00	08-06-07	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	127.47 79.67		
	1	T14A HIP GIRDER	8.00 0.00	14-07-00	04-09-00	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-08-02	59.57 38.17		
	1	T15A COMMON	8.00 0.00	14-07-00	06-04-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-08-02	61.52 38.33		
	3	T16 SCISSOR	8.00	08-02-00	04-01-08	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	110.91		
			6.00							75.00		
	1	T17 COMMON	8.00 0.00	08-02-00	04-01-08	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	35.70 22.67		



Delivery Shiplist

DATE	08/25/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 284266 LOCATION: WATERDOWN
 BUILDER: GREENPARK - RUSSELL GARDENS PH2 SUB-BUILDER:
 MODEL: ROSEWOOD 3 ELEVATION: 2

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	T22 MONOPITCH	8.00 8.00	03-09-00	03-10-13	2 X 4	2 X 4	00-00-00 00-00-00	01-04-13 01-10-13	18.24 12.67		
	1	T22C MONOPITCH	8.00 8.00	03-09-00	03-07-11	2 X 4	2 X 4	00-00-00 00-00-00	01-04-13 01-07-11	18.07 12.67		
	4	T18 MONOPITCH	4.00 0.00	08-07-00	03-04-08	2 X 4	2 X 4	01-03-08 00-00-00	00-06-03 00-03-08	129.24 83.32		
	4	P3 PIGGYBACK	8.00 0.00	02-11-13	01-04-06	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 00-04-07	38.12 24.00		
	15	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	251.85 160.05		
	2	J2 JACK-OPEN	6.00 0.00	05-10-08	02-00-12	2 X 4	2 X 4	01-03-08 -04-01-01	01-02-00 03-09-05	32.32 20.66		
	2	J3 JACK-OPEN	6.00 0.00	05-10-08	03-00-12	2 X 4	2 X 4	01-03-08 -02-01-01	01-02-00 03-09-05	37.42 23.34		
	2	J4 JACK-OPEN	6.00 0.00	01-10-08	02-00-12	2 X 4	2 X 4	01-03-08 -00-01-01	01-02-00 01-09-05	16.86 11.34		
	2	J5 JACK-OPEN	6.00 0.00	01-10-08	03-00-12	2 X 4	2 X 4	01-03-08 01-10-15	01-02-00 00-04-11	21.96 14.00		
	5	J6 JACK-OPEN	10.00 0.00	03-10-08	04-10-07	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 04-10-07	68.75 43.35		
	4	J7 JACK-OPEN	12.00 0.00	02-10-08	04-09-00	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 04-09-00	48.00 32.00		
	4	J10 JACK-OPEN	10.00 0.00	03-11-00	03-09-14	2 X 4	2 X 4	01-03-08 00-00-00	00-06-11 03-09-14	51.48 34.68		
	6	J11 JACK-OPEN	10.00 0.00	02-07-00	02-08-08	2 X 4	2 X 4	01-03-08 00-00-00	00-06-11 00-09-02	56.22 40.02		

TOTAL # TRUSS= 79.00 TOTAL BFT OF ALL TRUSSES= 2708.78 BFT. TOTAL WEIGHT OF ALL TRUSSES= 4250.36 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	HGUS26-2	
1	Hangers	HUSC26-2	
2	Hangers	LUS24-2	
4	Hangers	LJS26DS	
3	Hangers	LUS24	

TOTAL # ITEMS= 11.00



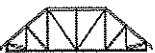
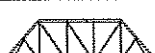
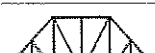
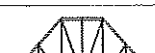
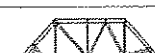
Delivery Shiplist

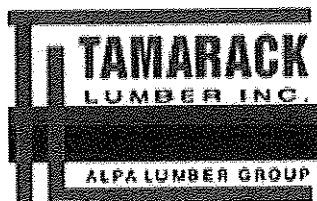
DATE	08/25/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 284267 LOCATION: WATERDOWN
 BUILDER: GREENPARK - RUSSELL GARDENS PH1 SUB-BUILDER:
 MODEL: ROSEWOOD 3 ELEVATION: 3

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	T1 HIP GIRDER	6.00	33-11-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-02-00 01-02-00	333.02		
	2 Ply		0.00							208.68		
	1	T2 HIP	6.00 0.00	33-11-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	133.28 84.67		
	1	T3 HIP	6.00 0.00	33-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	135.85 85.67		
	1	T4 HIP	6.00 0.00	33-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	141.99 89.17		
	1	T5 HIP	6.00 0.00	33-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	143.06 91.00		
	1	T6 HIP GIRDER	8.00	33-11-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-04-13 01-04-13	361.72		
	2 Ply		0.00							224.00		
	4	T70S PIGGYBACK	8.00 0.00	33-11-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	621.88 401.32		
	1	T7 HIP	8.00 0.00	33-11-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	137.63 89.83		
	1	T8 HIP	8.00 0.00	33-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	141.70 91.50		
	1	T9 HIP	8.00 0.00	33-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	148.63 95.50		
	1	T10 HIP	8.00 0.00	33-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	149.48 94.33		
	3	T11 HIP	8.00 0.00	33-11-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	467.85 295.50		
	1	T12 HIP GIRDER	8.00	27-07-00	06-10-07	2 X 6	2 X 6	01-03-08 01-05-00	01-04-13 01-04-13	180.28		
			0.00							110.67		
	1	T13A HIP	8.00 0.00	27-02-00	08-06-07	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 03-08-02	124.79 78.67		
	1	T14A HIP GIRDER	8.00 0.00	14-07-00	04-09-00	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-08-02	59.57 38.17		
	1	T15A COMMON	8.00 0.00	14-07-00	06-04-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-08-02	61.52 38.33		
	3	T16 SCISSOR	8.00 6.00	08-02-00	04-01-08	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	110.91 75.00		
	1	T17 COMMON	8.00 0.00	08-02-00	04-01-08	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	35.70 22.67		



Delivery Shiplist

DATE	08/25/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 284267 LOCATION: WATERDOWN
 BUILDER: GREENPARK - RUSSELL GARDENS PH1 SUB-BUILDER:
 MODEL: ROSEWOOD 3 ELEVATION: 3

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY PLY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER TOP BOT	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	1	P1 PIGGYBACK	8.00 0.00	05-11-13	02-01-00	2 X 4 2 X 4	00-00-00 00-00-00	00-04-07 00-04-07	20.58 13.00		
	3	P2 PIGGYBACK	8.00 0.00	05-11-13	02-04-06	2 X 4 2 X 4	00-00-00 00-00-00	00-04-07 00-04-07	55.41 35.01		
	15	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4 2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	251.85 160.05		
	2	J2 JACK-OPEN	6.00 0.00	05-10-08	02-00-12	2 X 4 2 X 4	01-03-08 -04-01-01	01-02-00 03-09-05	32.32 20.66		
	2	J3 JACK-OPEN	6.00 0.00	05-10-08	03-00-12	2 X 4 2 X 4	01-03-08 -02-01-01	01-02-00 03-09-05	37.42 23.34		
	2	J4 JACK-OPEN	6.00 0.00	01-10-08	02-00-12	2 X 4 2 X 4	01-03-08 -00-01-01	01-02-00 01-09-05	16.86 11.34		
	2	J5 JACK-OPEN	6.00 0.00	01-10-08	03-00-12	2 X 4 2 X 4	01-03-08 01-10-15	01-02-00 00-04-11	21.96 14.00		
	3	J6 JACK-OPEN	10.00 0.00	03-10-08	04-10-07	2 X 4 2 X 4	01-03-08 00-00-00	01-07-11 04-10-07	41.25 26.01		
	4	J6A JACK-OPEN	10.00 0.00	03-05-08	04-10-07	2 X 4 2 X 4	00-00-00 00-00-00	01-11-14 04-10-07	44.52 29.32		
	4	J7 JACK-OPEN	12.00 0.00	02-10-08	04-09-00	2 X 4 2 X 4	01-03-08 00-00-00	01-10-08 04-09-00	48.00 32.00		
	4	J10 JACK-OPEN	10.00 0.00	03-11-00	03-09-14	2 X 4 2 X 4	01-03-08 00-00-00	00-06-11 00-09-02	51.48 34.68		
	3	J11 JACK-OPEN	10.00 0.00	02-07-00	02-08-08	2 X 4 2 X 4	01-03-08 00-00-00	00-06-11 00-09-02	28.11 20.01		
	5	J12 JACK-OPEN	4.00 0.00	05-11-00	02-05-14	2 X 4 2 X 4	01-03-08 00-00-00	00-06-03 02-05-14	82.60 50.00		

TOTAL # TRUSS= 77.00

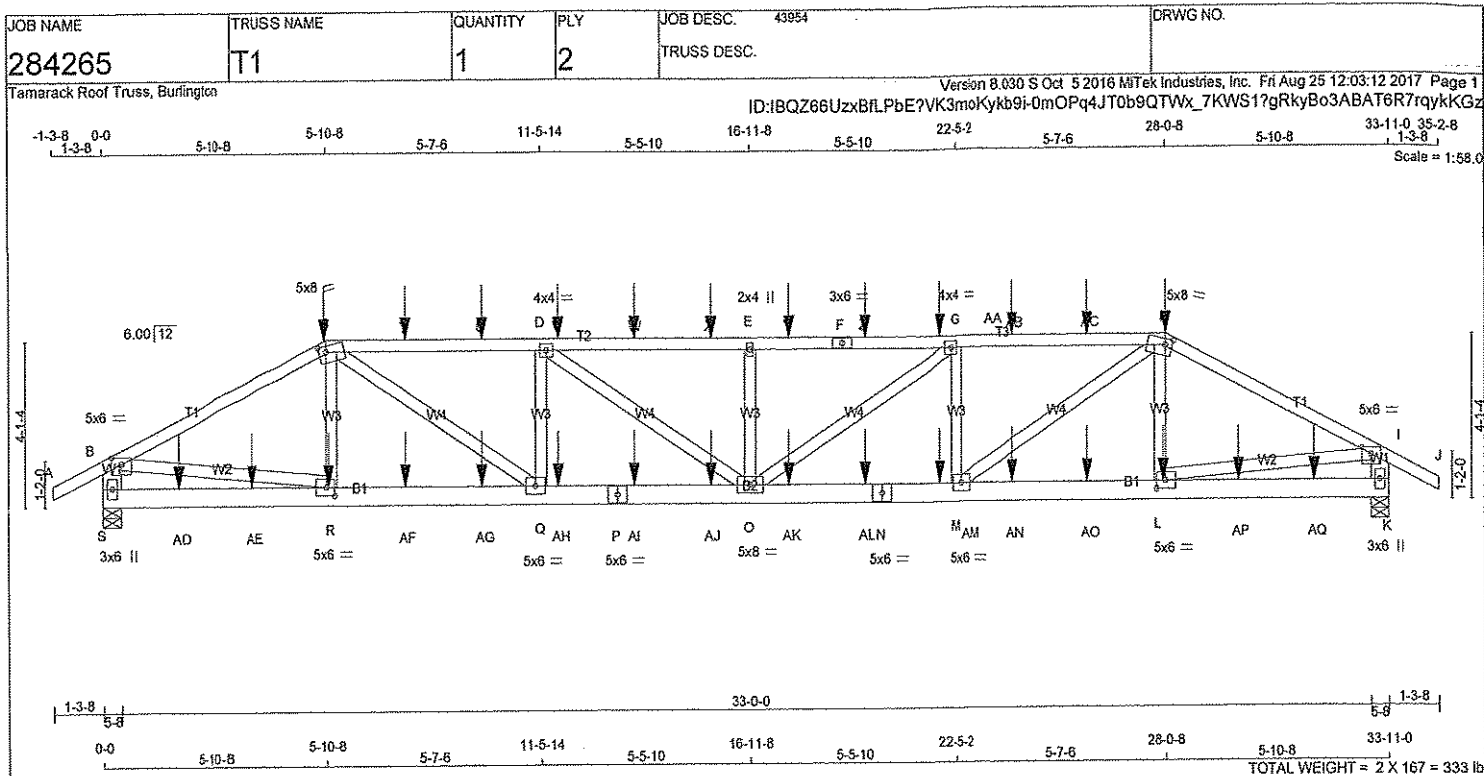
TOTAL BFT OF ALL TRUSSES=

2684.10 BFT. TOTAL WEIGHT OF ALL TRUSSES= 4221.22 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	HGUS26-2	
1	Hangers	HUSC26-2	
1	Hangers	LUS24-2	
3	Hangers	LJS26DS	
5	Hangers	LUS24	

TOTAL # ITEMS= 11.00



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - J	2x4	DRY	No.2
S - B	2x6	DRY	No.2
K - I	2x6	DRY	No.2
S - P	2x6	DRY	No.2
P - N	2x6	DRY	No.2
N - K	2x6	DRY	No.2

ALL WEBS 2x4 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	12	SIDE(61.0)
C - F	12	SIDE(61.0)
F - H	12	SIDE(61.0)
H - J	12	SIDE(61.0)
S - B	2	TOP
K - I	2	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
S - P	2	SIDE(183.1)
P - N	2	SIDE(183.1)
N - K	2	SIDE(183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
R - C	1	SIDE(1.2)
L - H	1	SIDE(1.2)
2x4	1	6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	2.00	3.00
C	TTWW-m	MT20	5.0	8.0	1.75	2.25
D	TMVW-t	MT20	4.0	4.0		
E	TMVW-w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMVW-t	MT20	4.0	4.0		
H	TTWW-m	MT20	5.0	8.0	1.75	2.25
I	TMVW-p	MT20	5.0	6.0	2.00	3.00
K	BMVW-t	MT20	3.0	6.0		
L	BMVW-t	MT20	5.0	6.0	2.50	2.75
M	BMVW-t	MT20	5.0	6.0		
N	BS-t	MT20	5.0	6.0		
O	BMVWV-t	MT20	5.0	8.0		
P	BS-t	MT20	5.0	6.0		
Q	BMVW-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	HORZ	BRG	IN-SX	BRG	IN-SX
S	3334	0	3334	0	0	5-8	5-8	5-8	5-8
K	3334	0	3334	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	2726	1530/0	608/0	0/0	0/0	588/0	0/0
K	2726	1530/0	608/0	0/0	0/0	588/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.06 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE	VERT. LOAD	LC1	MEMB.	FORCE	VERT. LOAD	LC1
FR-TO				FR-TO			
A-B	0/25	-84.3	-84.3	0.06 (1)	10.00	R-C	-268/274
B-C	-5085/0	-84.3	-84.3	0.49 (1)	3.91	C-Q	0/2811
C-T	-6860/0	-84.3	-84.3	0.63 (1)	3.25	Q-D	-1298/0
T-U	-6860/0	-84.3	-84.3	0.63 (1)	3.25	D-O	0/618
U-D	-6860/0	-84.3	-84.3	0.63 (1)	3.25	O-E	-676/0
D-V	-7536/0	-84.3	-84.3	0.67 (1)	3.06	E-G	0/618
V-W	-7536/0	-84.3	-84.3	0.67 (1)	3.06	M-G	-1298/0
W-X	-7536/0	-84.3	-84.3	0.67 (1)	3.06	M-H	0/2811
X-E	-7536/0	-84.3	-84.3	0.67 (1)	3.06	L-H	-268/274
E-Y	-7536/0	-84.3	-84.3	0.67 (1)	3.06	B-R	0/4565
Y-F	-7536/0	-84.3	-84.3	0.67 (1)	3.06	L-I	0/4565
F-Z	-7536/0	-84.3	-84.3	0.67 (1)	3.06		
Z-AA	-7536/0	-84.3	-84.3	0.67 (1)	3.06		
AA-G	-7536/0	-84.3	-84.3	0.67 (1)	3.06		
G-AB	-6860/0	-84.3	-84.3	0.63 (1)	3.25		
AB-AC	-6860/0	-84.3	-84.3	0.63 (1)	3.25		
AC-H	-6860/0	-84.3	-84.3	0.63 (1)	3.25		
H-I	-5085/0	-84.3	-84.3	0.49 (1)	3.91		
I-J	0/26	-84.3	-84.3	0.06 (1)	10.00		
S-B	-3215/0	0.0	0.0	0.11 (1)	7.72		
K-I	-3215/0	0.0	0.0	0.11 (1)	7.72		
S-AD	0/0	-28.0	-28.0	0.11 (2)	10.00		
AD-AE	0/0	-28.0	-28.0	0.11 (2)	10.00		
AE-R	0/0	-28.0	-28.0	0.11 (2)	10.00		
R-AF	0/4517	-28.0	-28.0	0.34 (1)	10.00		
AF-AG	0/4517	-28.0	-28.0	0.34 (1)	10.00		
AG-Q	0/4517	-28.0	-28.0	0.34 (1)	10.00		
Q-AH	0/6860	-28.0	-28.0	0.52 (1)	10.00		
AH-P	0/6860	-28.0	-28.0	0.52 (1)	10.00		
P-AI	0/6860	-28.0	-28.0	0.52 (1)	10.00		
AI-AJ	0/6860	-28.0	-28.0	0.52 (1)	10.00		
AJ-O	0/6860	-28.0	-28.0	0.52 (1)	10.00		
O-AK	0/6860	-28.0	-28.0	0.52 (1)	10.00		
AK-AL	0/6860	-28.0	-28.0	0.52 (1)	10.00		
AL-N	0/6860	-28.0	-28.0	0.52 (1)	10.00		
N-AM	0/6860	-28.0	-28.0	0.52 (1)	10.00		
AM-M	0/6860	-28.0	-28.0	0.52 (1)	10.00		
M-AN	0/4517	-28.0	-28.0	0.34 (1)	10.00		
AN-AO	0/4517	-28.0	-28.0	0.34 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = 1/360 (1.13")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.25")
ALLOWABLE DEFL.(TL) = 1/360 (1.13")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.41")

CSI: TC=0.67 (E-G:1), BC=0.52 (M-O:1),
WB=0.40 (B-R:1), SS=0.16 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

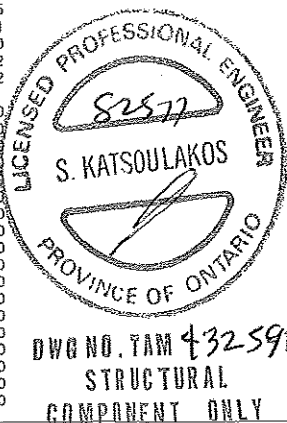
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.99 (L) (INPUT = 0.90)
JSI METAL= 0.58 (N) (INPUT = 1.00)



DWG NO. TAM 432-5917
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
R	BMWW-t	MT20	5.0	6.0	2.50	2.75
S	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 472.3 lbs FACTORED DOWN AT 5-10-8, 101.4 lbs FACTORED DOWN AT 7-11-4, 101.4 lbs FACTORED DOWN AT 9-11-4, 101.4 lbs FACTORED DOWN AT 11-11-4, 101.4 lbs FACTORED DOWN AT 13-11-4, 101.4 lbs FACTORED DOWN AT 15-11-4, 101.4 lbs FACTORED DOWN AT 17-11-12, 101.4 lbs FACTORED DOWN AT 19-11-12, 101.4 lbs FACTORED DOWN AT 21-11-12, 101.4 lbs FACTORED DOWN AT 23-11-12, AND 101.4 lbs FACTORED DOWN AT 25-11-12, AND 472.3 lbs FACTORED DOWN AT 28-0-8 ON TOP CHORD, AND 84.6 lbs FACTORED DOWN AT 1-11-4, 84.6 lbs FACTORED DOWN AT 3-11-4, 69.9 lbs FACTORED DOWN AT 5-11-4, 69.9 lbs FACTORED DOWN AT 7-11-4, 69.9 lbs FACTORED DOWN AT 9-11-4, 69.9 lbs FACTORED DOWN AT 11-11-4, 69.9 lbs FACTORED DOWN AT 13-11-4, 69.9 lbs FACTORED DOWN AT 15-11-4, 69.9 lbs FACTORED DOWN AT 17-11-12, 69.9 lbs FACTORED DOWN AT 19-11-12, 69.9 lbs FACTORED DOWN AT 21-11-12, 69.9 lbs FACTORED DOWN AT 23-11-12, 69.9 lbs FACTORED DOWN AT 25-11-12, 69.9 lbs FACTORED DOWN AT 27-11-12, AND 84.6 lbs FACTORED DOWN AT 29-11-12, AND 84.6 lbs FACTORED DOWN AT 31-11-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

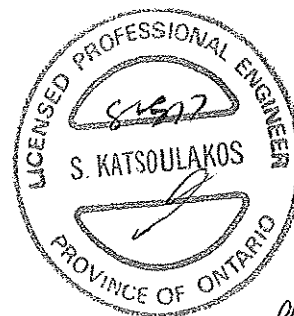
LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)
FR-TO		FROM	TO	FR-TO		FROM	TO
AO-L	0/4517	-28.0	-28.0 0.34 (1)	10.00			
L-AP	0/0	-28.0	-28.0 0.11 (2)	10.00			
AP-AQ	0/0	-28.0	-28.0 0.11 (2)	10.00			
AQ-K	0/0	-28.0	-28.0 0.11 (2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	5-10-8	-472	-472	---	FRONT	VERT	TOTAL
H	28-0-8	-472	-472	---	FRONT	VERT	TOTAL
L	27-11-12	-40	-70	---	FRONT	VERT	TOTAL
R	5-11-4	-40	-70	---	FRONT	VERT	TOTAL
T	7-11-4	-101	-101	---	FRONT	VERT	TOTAL
U	9-11-4	-101	-101	---	FRONT	VERT	TOTAL
V	11-11-4	-101	-101	---	FRONT	VERT	TOTAL
W	13-11-4	-101	-101	---	FRONT	VERT	TOTAL
X	15-11-4	-101	-101	---	FRONT	VERT	TOTAL
Y	17-11-12	-101	-101	---	FRONT	VERT	TOTAL
Z	19-11-12	-101	-101	---	FRONT	VERT	TOTAL
AA	21-11-12	-101	-101	---	FRONT	VERT	TOTAL
AB	23-11-12	-101	-101	---	FRONT	VERT	TOTAL
AC	25-11-12	-101	-101	---	FRONT	VERT	TOTAL
AD	1-11-4	-48	-85	---	FRONT	VERT	TOTAL
AE	3-11-4	-48	-85	---	FRONT	VERT	TOTAL
AF	7-11-4	-40	-70	---	FRONT	VERT	TOTAL
AG	9-11-4	-40	-70	---	FRONT	VERT	TOTAL
AH	11-11-4	-40	-70	---	FRONT	VERT	TOTAL
AI	13-11-4	-40	-70	---	FRONT	VERT	TOTAL
AJ	15-11-4	-40	-70	---	FRONT	VERT	TOTAL
AK	17-11-12	-40	-70	---	FRONT	VERT	TOTAL
AL	19-11-12	-40	-70	---	FRONT	VERT	TOTAL
AM	21-11-12	-40	-70	---	FRONT	VERT	TOTAL
AN	23-11-12	-40	-70	---	FRONT	VERT	TOTAL
AO	25-11-12	-40	-70	---	FRONT	VERT	TOTAL
AP	29-11-12	-48	-85	---	FRONT	VERT	TOTAL
AQ	31-11-12	-48	-85	---	FRONT	VERT	TOTAL

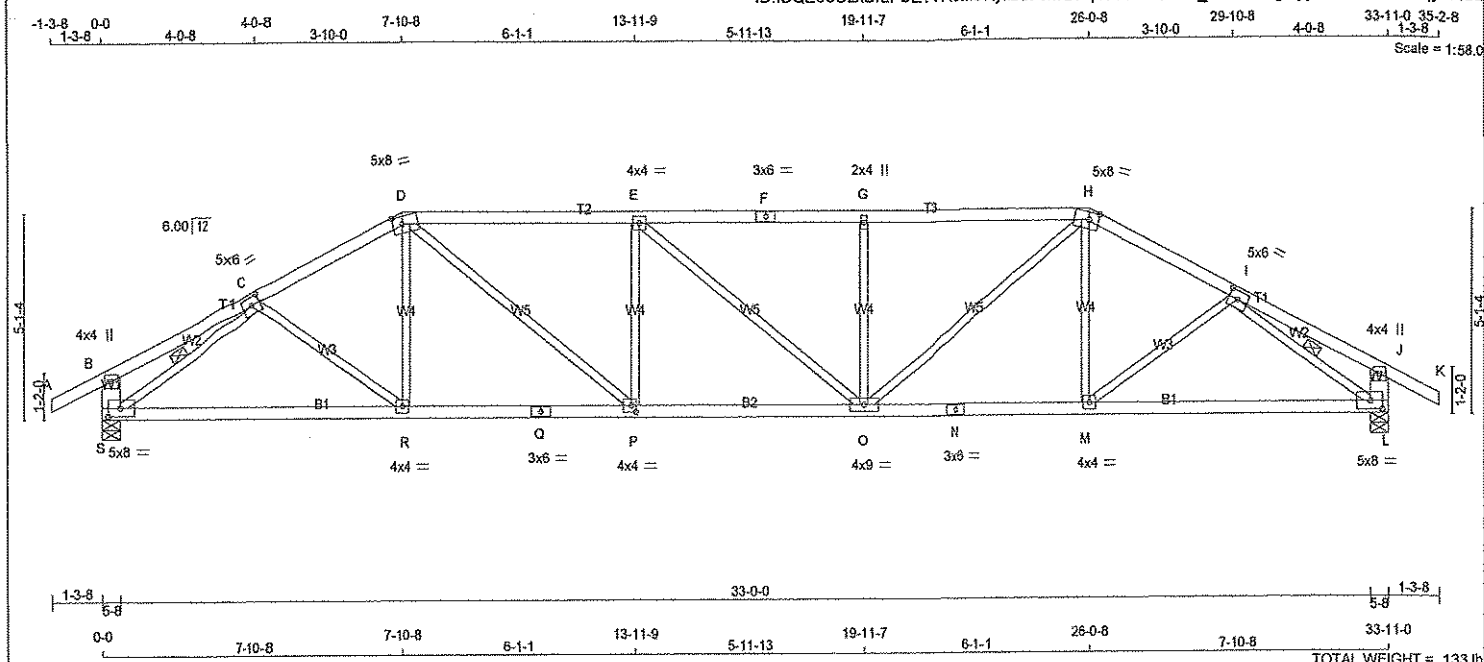


DRWG NO. TAM 43259-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
284265	T2	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 Mitek Industries, Inc. Fri Aug 25 12:03:12 2017 Page 1
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TOTAL WEIGHT = 133 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
S - B	2x6	DRY	No.2
L - J	2x6	DRY	No.2
S - Q	2x4	DRY	No.2
Q - N	2x4	DRY	No.2
N - L	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-I	MT20	5.0	6.0	2.50	2.75
D	TTWW-m	MT20	5.0	8.0	2.25	3.00
E	TMWW-I	MT20	4.0	4.0		
F	TS-I	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	8.0	2.25	3.00
I	TMWW-I	MT20	5.0	6.0	2.50	2.75
J	TMV+p	MT20	4.0	4.0		
L	BMVW-I	MT20	5.0	8.0	2.50	3.75
M	BMWW-I	MT20	4.0	4.0		
N	BS-I	MT20	3.0	6.0		
O	BMWW-I	MT20	4.0	9.0		
P	BMWW-I	MT20	4.0	4.0	2.00	1.50
Q	BS-I	MT20	3.0	6.0		
R	BMWW-I	MT20	4.0	4.0		
S	BMVW-I	MT20	5.0	8.0	2.50	3.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	UPLIFT
S	2018	0	2018	0
L	2018	0	2018	0

UNFACTORED REACTIONS

	1ST LCASE	MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1641	937 / 0	358 / 0	0 / 0	0 / 0	347 / 0	0 / 0
L	1641	937 / 0	358 / 0	0 / 0	0 / 0	347 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-S, I-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 26	-84.3 -84.3	0.11 (1)	10.00	C-R	0 / 210	0.05 (3)
B-C	0 / 14	-84.3 -84.3	0.17 (1)	10.00	R-D	0 / 288	0.08 (2)
C-D	-2730 / 0	-84.3 -84.3	0.28 (1)	3.97	D-P	0 / 1135	0.26 (1)
D-E	-3322 / 0	-84.3 -84.3	0.72 (1)	3.25	P-E	-551 / 0	0.21 (1)
E-F	-3320 / 0	-84.3 -84.3	0.71 (1)	3.25	E-O	-2 / 0	0.00 (1)
F-G	-3320 / 0	-84.3 -84.3	0.71 (1)	3.25	O-G	-551 / 0	0.21 (1)
G-H	-3320 / 0	-84.3 -84.3	0.72 (1)	3.25	O-H	0 / 1134	0.26 (1)
H-I	-2730 / 0	-84.3 -84.3	0.28 (1)	3.97	M-H	0 / 288	0.08 (2)
I-J	0 / 14	-84.3 -84.3	0.17 (1)	10.00	M-I	0 / 210	0.05 (3)
J-K	0 / 26	-84.3 -84.3	0.11 (1)	10.00	S-C	-2876 / 0	0.58 (1)
S-B	-249 / 0	0.0 0.0	0.02 (1)	7.81	I-L	-2876 / 0	0.58 (1)
L-J	-249 / 0	0.0 0.0	0.02 (1)	7.81			
S-R	0 / 2337	-28.0 -28.0	0.65 (2)	10.00			
R-Q	0 / 2430	-28.0 -28.0	0.67 (2)	10.00			
Q-P	0 / 2430	-28.0 -28.0	0.67 (2)	10.00			
P-O	0 / 3322	-28.0 -28.0	0.63 (1)	10.00			
O-N	0 / 2430	-28.0 -28.0	0.68 (2)	10.00			
N-M	0 / 2430	-28.0 -28.0	0.68 (2)	10.00			
M-L	0 / 2337	-28.0 -28.0	0.65 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.13")
CALCULATED VERT. DEFL.(LL)= L/999 (0.24")
ALLOWABLE DEFL.(TL)= L/360 (1.13")
CALCULATED VERT. DEFL.(TL)= L/999 (0.40")

CSF: TC=0.72 (D-E:1), BC=0.68 (M-O:2),
WB=0.58 (I-L:1), SS=0.24 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

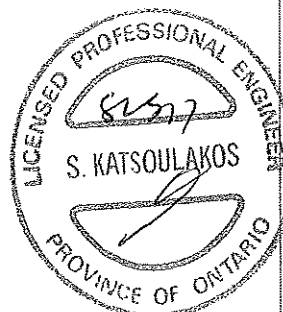
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

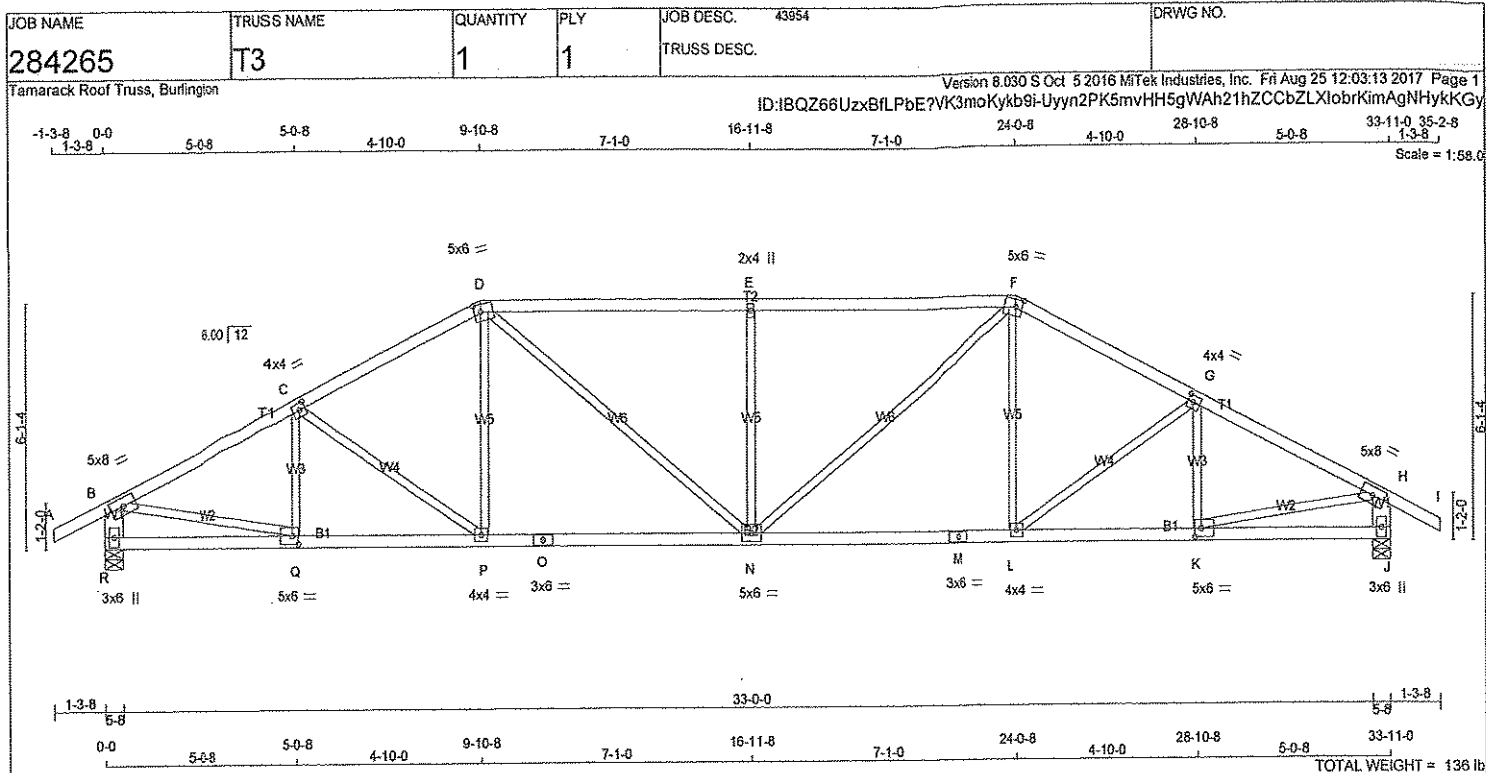
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SSI GRIP= 0.89 (L) (INPUT = 0.90)
SSI METAL= 0.72 (L) (INPUT = 1.00)



DRWG NO. TAM 4326017
STRUCTURAL
COMPONENT ONLY



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
R - B	2x6	DRY	No.2
J - H	2x6	DRY	No.2
R - O	2x4	DRY	No.2
O - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-1	MT20	5.0	8.0		
C	TMVW-1	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMVW-w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMVW-1	MT20	4.0	4.0	2.00	1.75
H	TMVW-1	MT20	5.0	8.0		
J	BMV1+p	MT20	3.0	6.0		
K	BMVW-1	MT20	5.0	6.0	2.50	2.00
L	BMVW-1	MT20	4.0	4.0		
M	BS-1	MT20	3.0	6.0		
N	BMVW-1	MT20	5.0	6.0		
O	BS-1	MT20	3.0	6.0		
P	BMVW-1	MT20	4.0	4.0		
Q	BMVW-1	MT20	5.0	6.0	2.50	2.00
R	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
		GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
R	2018	0	2018	0	0
J	2018	0	2018	0	0

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1641	937 / 0	356 / 0	0 / 0	0 / 0	347 / 0	0 / 0
J	1641	937 / 0	356 / 0	0 / 0	0 / 0	347 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.34 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LC1 (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 26	-84.3 -84.3	0.11 (1)	Q-C	-289 / 42	0.08 (1)	
B-C	-2770 / 0	-84.3 -84.3	0.37 (1)	C-P	-228 / 0	0.14 (1)	
C-D	-2603 / 0	-84.3 -84.3	0.34 (1)	P-D	0 / 376	0.09 (2)	
D-E	-2883 / 0	-84.3 -84.3	0.73 (1)	D-N	0 / 738	0.17 (1)	
E-F	-2883 / 0	-84.3 -84.3	0.73 (1)	N-E	-734 / 0	0.43 (1)	
F-G	-2803 / 0	-84.3 -84.3	0.34 (1)	N-F	0 / 738	0.17 (1)	
G-H	-2770 / 0	-84.3 -84.3	0.37 (1)	L-F	0 / 376	0.09 (2)	
H-I	0 / 26	-84.3 -84.3	0.11 (1)	L-G	-228 / 0	0.14 (1)	
R-B	-1954 / 0	0.0 0.0	0.13 (1)	K-G	-289 / 42	0.08 (1)	
J-H	-1954 / 0	0.0 0.0	0.13 (1)	B-Q	0 / 2532	0.57 (1)	
				K-H	0 / 2532	0.57 (1)	
R-Q	0 / 0	-28.0 -28.0	0.15 (2)				
Q-P	0 / 2496	-28.0 -28.0	0.52 (1)				
P-O	0 / 2313	-28.0 -28.0	0.56 (2)				
O-N	0 / 2313	-28.0 -28.0	0.56 (2)				
N-M	0 / 2313	-28.0 -28.0	0.56 (2)				
M-L	0 / 2313	-28.0 -28.0	0.56 (2)				
L-K	0 / 2496	-28.0 -28.0	0.52 (1)				
K-J	0 / 0	-28.0 -28.0	0.15 (2)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 3.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 46.1	PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.13")
CALCULATED VERT. DEFL.(LL)= U/999 (0.19")
ALLOWABLE DEFL.(TL)= L/360 (1.13")
CALCULATED VERT. DEFL.(TL)= U/999 (0.32")

CSI: TC=0.73 (D-E:1), BC=0.56 (L-N:2), WB=0.57 (H-K:1), SSI=0.29 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1667
	822	2284	1656

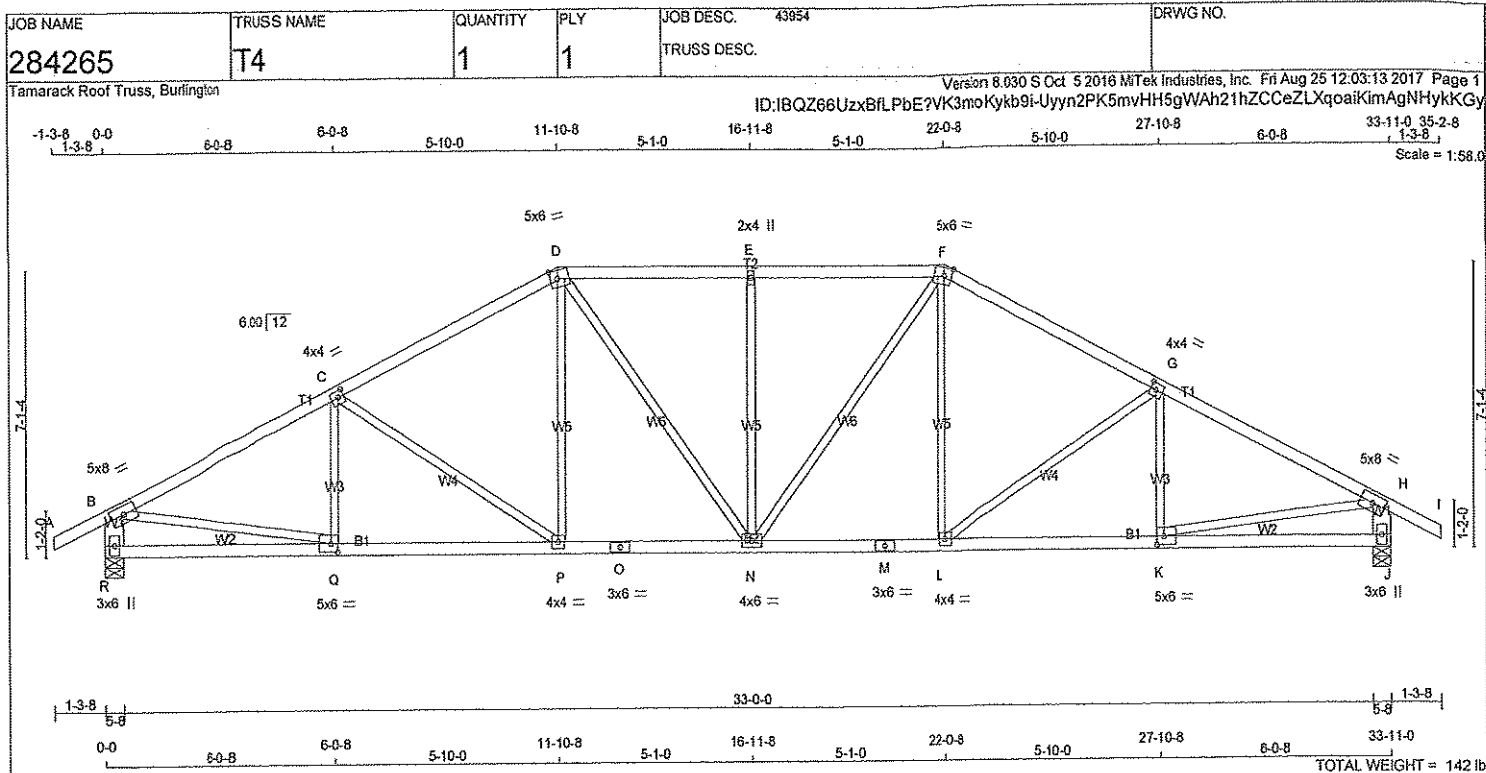
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (D) (INPUT = 0.90)
JSI METAL= 0.62 (Q) (INPUT = 1.00)



DWG NO. TAM 43261-17
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
R - B	2x6	DRY	No.2
J - H	2x6	DRY	No.2
R - O	2x4	DRY	No.2
O - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

DRY: SEASONED LUMBER

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0		
C	TMVW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	6.0	2.50	2.25
E	TMVW-w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.50	2.25
G	TMVW-t	MT20	4.0	4.0	2.00	1.75
H	TMVW-t	MT20	5.0	8.0		
J	BMV1+p	MT20	3.0	6.0		
K	BMVW-t	MT20	5.0	6.0	2.50	2.25
L	BMVW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMVW-w	MT20	4.0	6.0		
O	BS-t	MT20	3.0	6.0		
P	BMVW-t	MT20	4.0	4.0		
Q	BMVW-t	MT20	5.0	6.0	2.50	2.25
R	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED		MAXIMUM FACTORED		INPUT	REQRD
	GROSS REACTION	DOWN	GROSS REACTION	DOWN		
JT	VERT	HORZ	DOWN	HORZ	BRG	BRG
R	2018	0	2018	0	5-8	5-8
J	2018	0	2018	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
R	1641	937 / 0	356 / 0	0 / 0	0 / 0	347 / 0	0 / 0
J	1641	937 / 0	356 / 0	0 / 0	0 / 0	347 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.67 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX. (LC)	MAX. UNBRACED LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LC1 MAX. (LC)
FR-TO					FR-TO		
A-B	0 / 26	-84.3	-84.3 0.11 (1)	10.00	Q-C	-179 / 152	0.05 (1)
B-C	-2821 / 0	-84.3	-84.3 0.54 (1)	3.67	C-P	-458 / 0	0.44 (1)
C-D	-2448 / 0	-84.3	-84.3 0.48 (1)	3.95	P-D	0 / 438	0.10 (2)
D-E	-2413 / 0	-84.3	-84.3 0.35 (1)	4.11	D-N	0 / 406	0.09 (1)
E-F	-2413 / 0	-84.3	-84.3 0.35 (1)	4.11	N-E	-521 / 0	0.46 (1)
F-G	-2448 / 0	-84.3	-84.3 0.48 (1)	3.95	N-F	0 / 408	0.09 (1)
G-H	-2821 / 0	-84.3	-84.3 0.54 (1)	3.67	L-F	0 / 438	0.10 (2)
H-I	0 / 26	-84.3	-84.3 0.11 (1)	10.00	L-G	-458 / 0	0.44 (1)
R-B	-1947 / 0	0.0	0.0 0.13 (1)	7.22	K-G	-179 / 152	0.05 (1)
J-H	-1947 / 0	0.0	0.0 0.13 (1)	7.22	B-Q	0 / 2572	0.58 (1)
					K-H	0 / 2572	0.58 (1)
R-Q	0 / 0	-28.0	-28.0 0.24 (3)	10.00			
Q-P	0 / 2546	-28.0	-28.0 0.53 (2)	10.00			
P-O	0 / 2170	-28.0	-28.0 0.44 (1)	10.00			
O-N	0 / 2170	-28.0	-28.0 0.44 (1)	10.00			
N-M	0 / 2170	-28.0	-28.0 0.44 (1)	10.00			
M-L	0 / 2170	-28.0	-28.0 0.44 (1)	10.00			
L-K	0 / 2546	-28.0	-28.0 0.53 (2)	10.00			
K-J	0 / 0	-28.0	-28.0 0.24 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.13")
CALCULATED VERT. DEFL.(LL) = L/999 (0.16")
ALLOWABLE DEFL.(TL) = L/360 (1.13")
CALCULATED VERT. DEFL.(TL) = L/999 (0.26")

CSI: TC=0.54 (G-H), BC=0.53 (P-Q), WB=0.58 (B-Q), SSI=0.22 (G-H)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

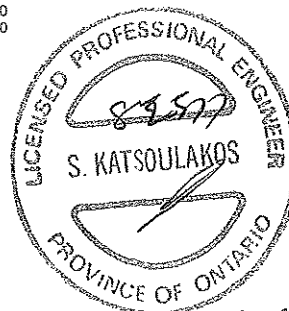
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

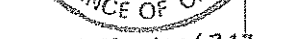
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL= 0.64 (Q) (INPUT = 1.00)



DRWG NO. TAM 43262-17
STRUCTURAL
COMPONENT ONLY


 DWG NO. TAM 4326317
 STRUCTURAL
 COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
284265	T6	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
P	BS-1	MT20	5.0	6.0		
Q	BMWW+1	MT20	4.0	6.0		
R	BMWW+1	MT20	4.0	6.0		
S	BS-1	MT20	5.0	6.0		
T	BMWWWW-1	MT20	8.0	12.0	4.25	6.00
U	BMWW-1	MT20	5.0	6.0	2.50	2.00
V	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 244.5 lbs FACTORED DOWN AT 4-0-11,
AND 104.9 lbs FACTORED DOWN AT 4-0-11,
AND 101.4 lbs FACTORED DOWN AT 6-1-6 ON
TOP CHORD, AND 69.9 lbs FACTORED DOWN
AT 2-1-6, 69.9 lbs FACTORED DOWN AT 4-1-6,
AND 69.9 lbs FACTORED DOWN AT 6-1-6, AND
2455.7 lbs FACTORED DOWN AT 6-7-8 ON
BOTTOM CHORD. DESIGN FOR UNSPECIFIED
CONNECTION(S) IS DELEGATED TO THE
BUILDING DESIGNER.



DWG NO. TAM 43264-17
STRUCTURAL
COMPONENT ONLY

Per

[A]

LUMBER				
N. L. G. A. RULES	CHORDS	SURFACE SPACING (IN)	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - J	2x4	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
V - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
V - S	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT T - D O - I	2x4 2x6 2x6	DRY DRY DRY	No.2 No.2 No.2	SPF SPF SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C	1 12	SIDE(61.0)
C-F	1 12	SIDE(61.0)
F-J	1 12	TOP
J-L	1 12	TOP
V-B	2 12	TOP
M-K	2 12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
V-S	2 12	SIDE(183.1)
S-P	2 12	TOP
P-M	2 12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x4	1 6	SIDE(469.6)
D-T	2 6	
2x6	2 6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-6 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-p	MT20	5.0	8.0	Edge	
C TTWW+m	MT20	7.0	8.0	Edge	2.50
D TMW+w	MT20	3.0	6.0		
E TMWW-t	MT20	4.0	6.0		
F TS-I	MT20	3.0	6.0		
G TMWW-t	MT20	4.0	4.0		
H TMWW-t	MT20	4.0	6.0		
I TMW+w	MT20	3.0	6.0		
J TTWW+m	MT20	7.0	8.0	Edge	2.50
K TMVW-p	MT20	5.0	8.0	Edge	
M BMV1+p	MT20	3.0	6.0		
N BMWW-t	MT20	5.0	6.0	2.50	2.00
O BMWWW-t	MT20	8.0	12.0	4.25	6.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
V	4545	0	4545	0	0	5-8	5-8	5-8	5-8
M	2566	0	2566	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS		PERM.LIVE WIND DEAD SOIL	
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND DEAD SOIL
V	3682	2123 / 0	785 / 0	0 / 0	0 / 0 774 / 0 0 / 0
M	2083	1195 / 0	448 / 0	0 / 0	0 / 0 439 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.30 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF H-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS	
MEMB.	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MEMB.	MAX. FORCE (LBS)
FR-TO		FROM TO		LENGTH FR-TO	
A-B	0 / 32	-84.3 -84.3	0.06 (1)	10.00	U-C -862 / 0
A-C	-5553 / 0	-84.3 -84.3	0.27 (1)	3.93	R-E -271 / 111
B-C	-6725 / 0	-84.3 -84.3	0.37 (1)	3.47	Q-H 0 / 789
W-D	-6725 / 0	-84.3 -84.3	0.37 (1)	3.47	N-J -514 / 0
D-E	-6726 / 0	-84.3 -84.3	0.64 (1)	3.44	B-U 0 / 4740
E-F	-7236 / 0	-84.3 -84.3	0.43 (1)	3.30	K-K 0 / 2517

JOB NAME 284266	TRUSS NAME T6Z	QUANTITY 1	PLY 2	JOB DESC. TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 Mitek Industries, Inc. Fri Aug 25 12:17:18 2017 Page 2
ID:1BQZ66Uzx8fLPbE?VK3moKykb9l-FWVVDOLZ8nXkNzXdah?irJPi0jzwCHn9i0kNuXoykK3

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
P	BS-t	MT20	5.0	6.0		
Q	BMWW-t	MT20	4.0	6.0		
R	BMWW-t	MT20	4.0	6.0		
S	BS-t	MT20	5.0	6.0		
T	BMWW-t	MT20	8.0	12.0	4.25	6.00
U	BMWW-t	MT20	5.0	6.0	2.50	2.00
V	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

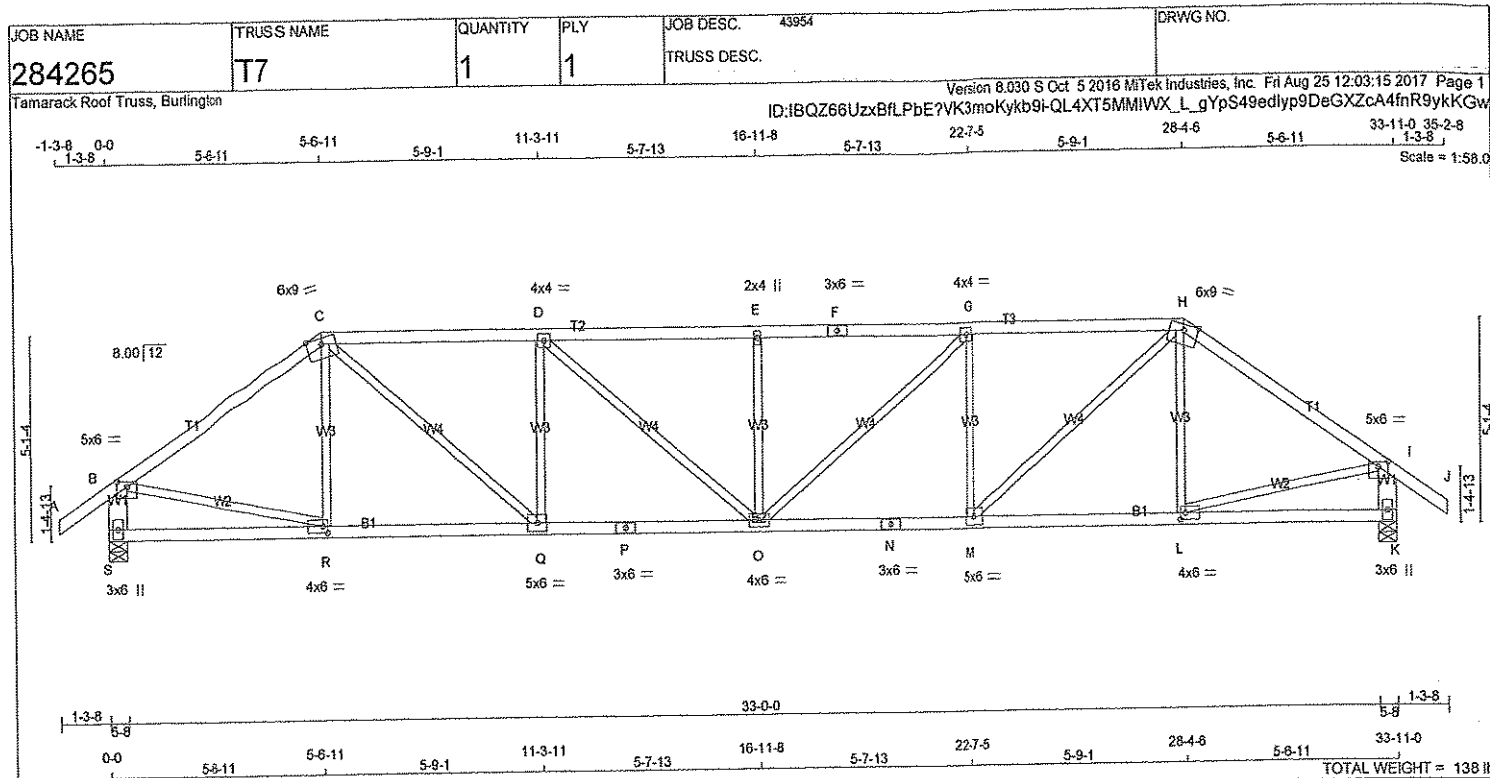
- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 244.5 lbs FACTORED DOWN AT 4-0-11, AND 104.9 lbs FACTORED DOWN AT 4-0-11, AND 101.4 lbs FACTORED DOWN AT 5-10-4 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 2-1-6, 69.9 lbs FACTORED DOWN AT 4-1-6, AND 69.9 lbs FACTORED DOWN AT 5-10-4, AND 2501.9 lbs FACTORED DOWN AT 6-2-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
X	2-1-6	-40	-70	—	BACK	VERT	TOTAL
Y	5-10-4	-40	-70	—	BACK	VERT	TOTAL



DWRND.TAM 4328017
STRUCTURAL
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LUMBER	N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - F	2x4	DRY	No.2	SPF	
F - H	2x4	DRY	No.2	SPF	
H - J	2x4	DRY	No.2	SPF	
S - B	2x6	DRY	No.2	SPF	
K - I	2x6	DRY	No.2	SPF	
S - P	2x4	DRY	No.2	SPF	
P - N	2x4	DRY	No.2	SPF	
N - K	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B TMVW-p	MT20	5.0	6.0	1.50	3.00		
C TTWW-m	MT20	6.0	9.0	Edge			
D TMWW-l	MT20	4.0	4.0				
E TMW-w	MT20	2.0	4.0				
F TS-t	MT20	3.0	6.0				
G TMWW-t	MT20	4.0	4.0				
H TTWW-m	MT20	6.0	9.0	Edge			
I TMVW-p	MT20	5.0	6.0	1.50	3.00		
K BMV1+p	MT20	3.0	6.0				
L BMWW-t	MT20	4.0	6.0	2.00	1.50		
M BMWW-t	MT20	5.0	6.0				
N BS-t	MT20	3.0	6.0				
O BMWW-t	MT20	4.0	6.0				
P BS-t	MT20	3.0	6.0				
Q BMWW-t	MT20	5.0	6.0				
R BMWW-t	MT20	4.0	6.0	2.00	1.50		
S BMV1+p	MT20	3.0	6.0				

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECD BRG
JT	VERT	DOWN	HORZ	UPLIFT
S	2019	0	2019	0
K	2019	0	2019	0

UNFACTORED REACTIONS	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
JT	COMBINED	SNOW	LIVE	PERM.LIVE	
S	1641	938 / 0	356 / 0	0 / 0	347 / 0
K	1641	938 / 0	356 / 0	0 / 0	347 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.23 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MAX. UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)
FR-TO						FR-TO			
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	R-C	-177 / 130	0.07 (1)	
B-C	-2247 / 0	-84.3	-84.3	0.65 (1)	3.91	C-Q	0 / 1547	0.35 (1)	
C-D	-3051 / 0	-84.3	-84.3	0.64 (1)	3.44	Q-D	-839 / 0	0.32 (1)	
D-E	-3403 / 0	-84.3	-84.3	0.68 (1)	3.23	D-O	0 / 461	0.10 (1)	
E-F	-3403 / 0	-84.3	-84.3	0.68 (1)	3.23	O-E	-439 / 0	0.17 (1)	
F-G	-3403 / 0	-84.3	-84.3	0.68 (1)	3.23	O-G	0 / 461	0.10 (1)	
G-H	-3051 / 0	-84.3	-84.3	0.64 (1)	3.44	M-G	-839 / 0	0.32 (1)	
H-I	-2247 / 0	-84.3	-84.3	0.65 (1)	3.91	M-H	0 / 1547	0.35 (1)	
I-J	0 / 32	-84.3	-84.3	0.11 (1)	10.00	L-H	-177 / 130	0.07 (1)	
S-B	-1956 / 0	0.0	0.0	0.13 (1)	7.20	B-R	0 / 1901	0.43 (1)	
K-I	-1956 / 0	0.0	0.0	0.13 (1)	7.20	L-I	0 / 1901	0.43 (1)	
S-R	0 / 0	-28.0	-28.0	0.22 (3)	10.00				
R-Q	0 / 1863	-28.0	-28.0	0.44 (2)	10.00				
Q-P	0 / 3051	-28.0	-28.0	0.57 (1)	10.00				
P-O	0 / 3051	-28.0	-28.0	0.57 (1)	10.00				
O-N	0 / 3051	-28.0	-28.0	0.57 (1)	10.00				
N-M	0 / 3051	-28.0	-28.0	0.57 (1)	10.00				
M-L	0 / 1863	-28.0	-28.0	0.44 (2)	10.00				
L-K	0 / 0	-28.0	-28.0	0.22 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.13")
CALCULATED VERT. DEFL.(LL) = L/999 (0.22")
ALLOWABLE DEFL.(TL) = L/360 (1.13")
CALCULATED VERT. DEFL.(TL) = L/999 (0.36")

CSI: TC=0.68 (D-E-1), BC=0.57 (O-Q-1), WB=0.43 (B-R-1), SSI=0.23 (C-D-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

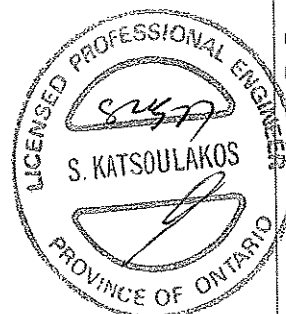
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1657 822 2284 1658

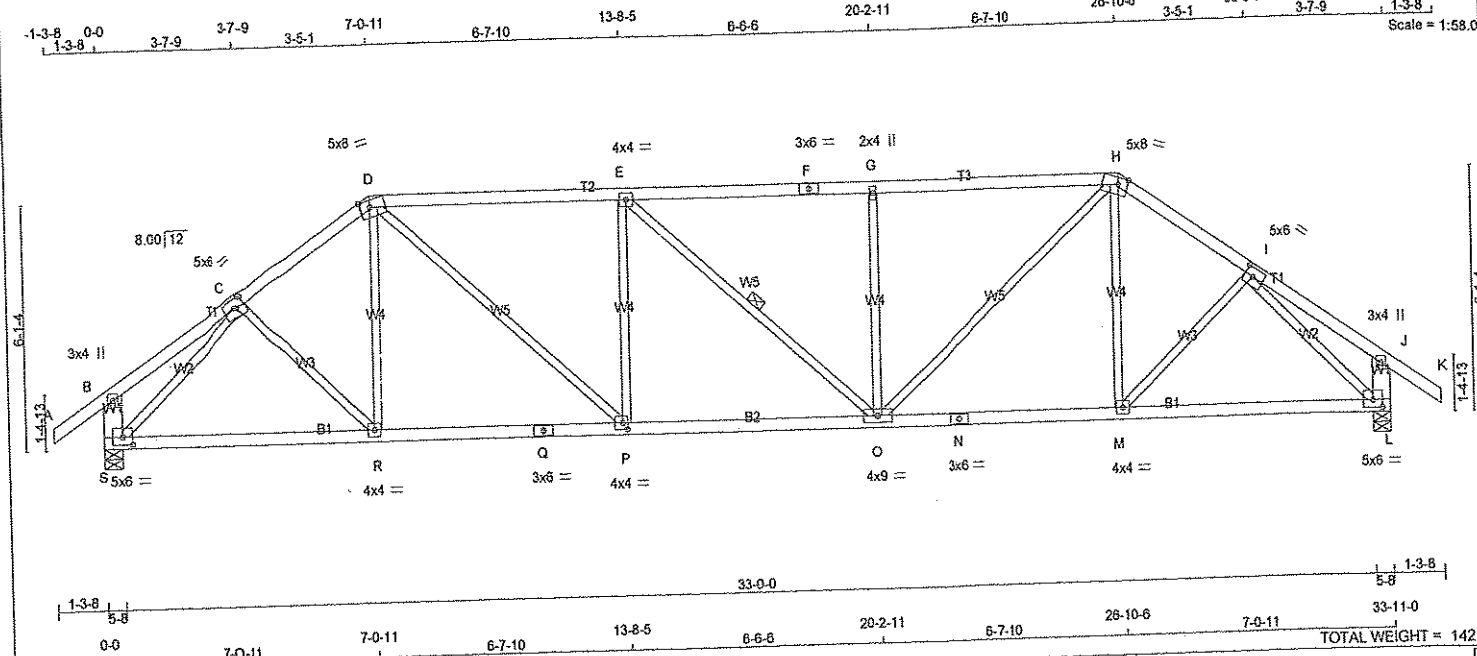
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.89 (P) (INPUT = 0.90)
SI METAL= 0.86 (N) (INPUT = 1.00)



DWG NO. TAM-4326517
STRUCTURAL
COMPONENT ONLY



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
S - B	2x6	DRY	No.2
L - J	2x6	DRY	No.2
S - Q	2x4	DRY	No.2
Q - N	2x4	DRY	No.2
N - L	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
SPF			

DRY: SEASONED LUMBER

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	3.0	4.0
C	TMVW-1	MT20	5.0	6.0 2.50 2.50
D	TTVW-m	MT20	5.0	8.0 2.00 3.00
E	TMVW-1	MT20	4.0	4.0
F	TS-1	MT20	3.0	6.0
G	TMVW-w	MT20	2.0	4.0
H	TTVW-m	MT20	5.0	8.0 2.00 3.00
I	TMVW-1	MT20	5.0	6.0 2.50 2.50
J	TMV+p	MT20	3.0	4.0
L	BMVW-1	MT20	5.0	6.0 2.25 3.00
M	BMVW-1	MT20	4.0	4.0
N	BS-1	MT20	3.0	6.0
O	BMVW-1	MT20	4.0	9.0
P	BMVW-1	MT20	4.0	4.0 2.00 1.50
Q	BS-1	MT20	3.0	6.0
R	BMVW-1	MT20	4.0	4.0
S	BMVW-1	MT20	5.0	6.0 2.25 3.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
FACTORED	MAXIMUM FACTORED	INPUT	RECORD
GROSS REACTION	GROSS REACTION	BRG	BRG
DOWN	DOWN	IN-SX	IN-SX
JT	2019 0	5-8	5-8
S	2019 0	5-8	5-8
L	2019 0	5-8	5-8

UNFACTORED REACTIONS						
1ST LCASE	MAX /MIN	COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
S	1641	938 / 0	356 / 0	0 / 0	0 / 0	347 / 0
L	1641	938 / 0	356 / 0	0 / 0	0 / 0	347 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

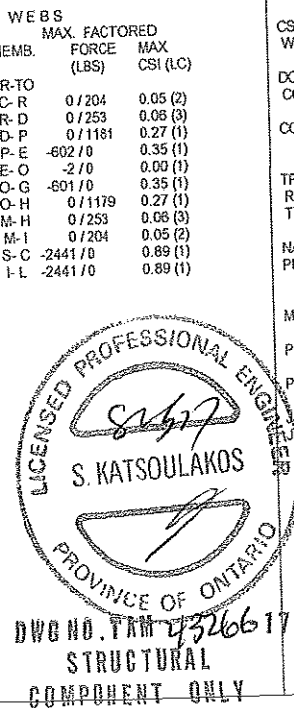
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.44 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

CHORDS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LBS)
FR-TO			
A-B	0 / 32	-84.3	-84.3 0.11 (1)
B-C	0 / 16	-84.3	-84.3 0.14 (1)
C-D	-2253 / 0	-84.3	-84.3 0.21 (1)
D-E	-2749 / 0	-84.3	-84.3 0.79 (1)
E-F	-2748 / 0	-84.3	-84.3 0.78 (1)
F-G	-2748 / 0	-84.3	-84.3 0.78 (1)
G-H	-2748 / 0	-84.3	-84.3 0.78 (1)
H-I	-2253 / 0	-84.3	-84.3 0.21 (1)
I-J	0 / 16	-84.3	-84.3 0.14 (1)
J-K	0 / 32	-84.3	-84.3 0.11 (1)
S-B	-235 / 0	0.0	0.0 0.02 (1)
L-J	-235 / 0	0.0	0.0 0.02 (1)
S-R	0 / 1749	-28.0	-28.0 0.53 (2)
R-Q	0 / 1860	-28.0	-28.0 0.56 (2)
Q-P	0 / 1860	-28.0	-28.0 0.56 (2)
P-O	0 / 2749	-28.0	-28.0 0.55 (1)
O-N	0 / 1860	-28.0	-28.0 0.56 (2)
N-M	0 / 1860	-28.0	-28.0 0.56 (2)
M-L	0 / 1749	-28.0	-28.0 0.53 (2)



DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPJC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.13")
CALCULATED VERT. DEFL. (LL) = L/999 (0.18")
ALLOWABLE DEFL. (TL) = L/360 (1.13")
CALCULATED VERT. DEFL. (TL) = L/999 (0.30")

CSI: TC=0.79 (D-E:1), BC=0.56 (M-O:2), WB=0.89 (I-L:1), SSI=0.26 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

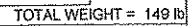
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SSI GRIP= 0.90 (P) (INPUT = 0.90)
SSI METAL= 0.61 (I) (INPUT = 1.00)

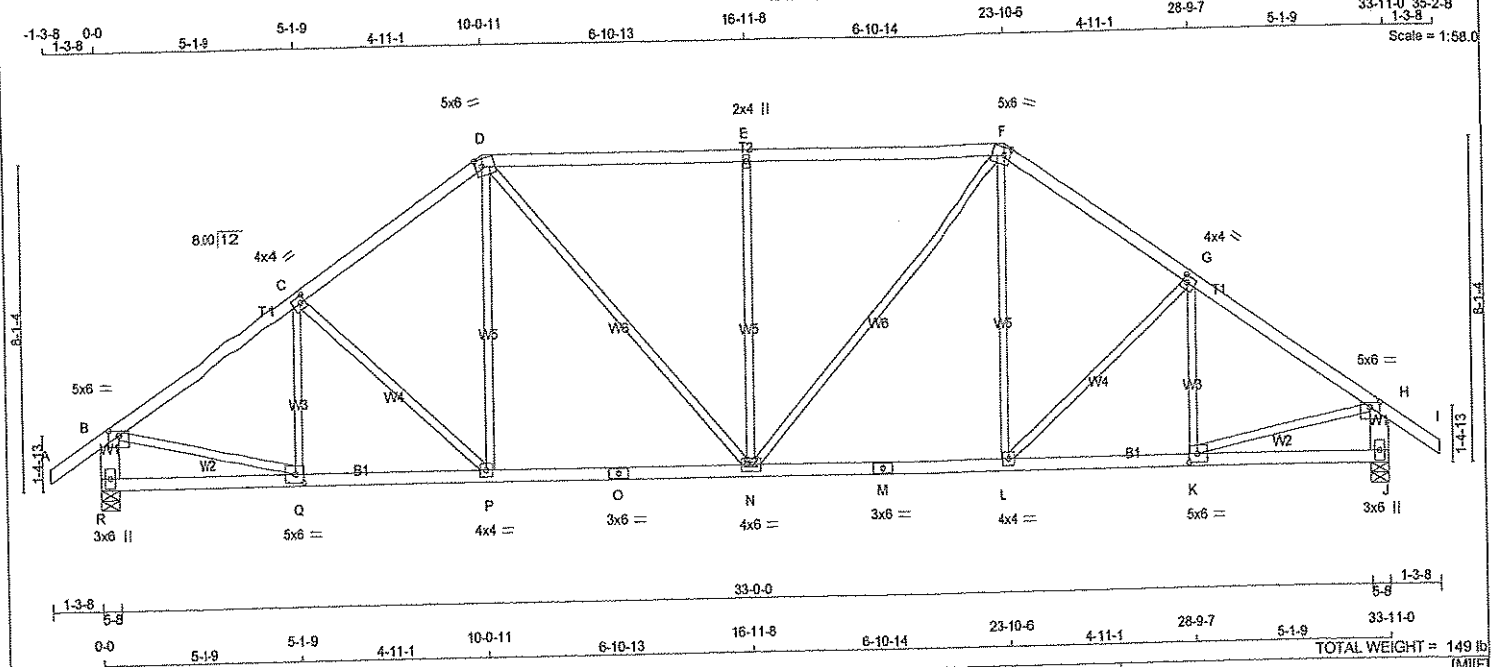
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SI GRIP= 0.88 (D) (INPUT = 0.90)
SI METAL= 0.61 (I) (INPUT = 1.00)

DWG NO. TAM 4326717
STRUCTURAL
COMPONENT ONLY

JOB NAME 284265	TRUSS NAME T10	QUANTITY 1	PLY 1	JOB DESC. 43954	DRWG NO.
Version 8.030 S Oct 5 2016 M/Tek Industries, Inc. Fri Aug 25 12:03:16 2017 Page 1					
Tamarack Roof Truss, Burlington					
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LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
R - B	2x6	DRY	No.2
J - H	2x6	DRY	No.2
R - O	2x4	DRY	No.2
O - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
DRY: SEASONED LUMBER			

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMVW-p	MT20	5.0	6.0 1.50 3.00
C	TMVW-t	MT20	4.0	4.0 2.00 1.50
D	TTWW-m	MT20	5.0	6.0 2.25 2.00
E	TMVW-w	MT20	2.0	4.0
F	TTWW-m	MT20	5.0	6.0 2.25 2.00
G	TMVW-t	MT20	4.0	4.0 2.00 1.50
H	TMVW-p	MT20	5.0	6.0 1.50 3.00
J	BMV1+p	MT20	3.0	6.0
K	BMVW-t	MT20	5.0	6.0 2.50 2.75
L	BMVW-t	MT20	4.0	4.0
M	BS-t	MT20	3.0	6.0
N	BMVW-t	MT20	4.0	6.0
O	BS-t	MT20	3.0	6.0
P	BMVW-t	MT20	4.0	4.0
Q	BMVW-t	MT20	5.0	6.0 2.50 2.75
R	BMV1+p	MT20	3.0	6.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQ'D
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG
R	2019	0	2019	0	5-8
J	2019	0	2019	0	5-8

UNFACTORED REACTIONS		MAX/MIN. COMPONENT REACTIONS					
JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1641	938 / 0	356 / 0	0 / 0	0 / 0	347 / 0	0 / 0
J	1641	938 / 0	356 / 0	0 / 0	0 / 0	347 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.96 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING		TOTAL LOAD CASES: (4)				WEBS		MAX. FACTORED	
CHORDS		FACTORED							
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)		
FR-TO		FROM TO			FR-TO				
A-B	0 / 32	-84.3	-84.3 0.11 (1)	10.00	Q-C	-258 / 64	0.09 (1)		
B-C	-2266 / 0	-84.3	-84.3 0.36 (1)	4.24	C-P	-233 / 0	0.20 (1)		
C-D	-2116 / 0	-84.3	-84.3 0.34 (1)	4.37	P-D	0 / 389	0.09 (2)		
D-E	-2142 / 0	-84.3	-84.3 0.61 (1)	3.96	D-N	0 / 609	0.14 (1)		
E-F	-2142 / 0	-84.3	-84.3 0.61 (1)	3.96	N-E	-714 / 0	0.90 (1)		
F-G	-2116 / 0	-84.3	-84.3 0.34 (1)	4.37	N-F	0 / 609	0.14 (1)		
G-H	-2266 / 0	-84.3	-84.3 0.36 (1)	4.24	L-F	0 / 389	0.09 (2)		
H-I	0 / 32	-84.3	-84.3 0.11 (1)	10.00	L-G	-233 / 0	0.20 (1)		
R-B	-1957 / 0	0.0	0.0 0.13 (1)	7.20	K-G	-256 / 64	0.09 (1)		
J-H	-1957 / 0	0.0	0.0 0.13 (1)	7.20	B-Q	0 / 1950	0.44 (1)		
					K-H	0 / 1950	0.44 (1)		
R-Q	0 / 0	-28.0	-28.0 0.15 (3)	10.00					
Q-P	0 / 1908	-28.0	-28.0 0.42 (1)	10.00					
P-O	0 / 1739	-28.0	-28.0 0.49 (2)	10.00					
O-N	0 / 1739	-28.0	-28.0 0.49 (2)	10.00					
N-M	0 / 1739	-28.0	-28.0 0.49 (2)	10.00					
M-L	0 / 1739	-28.0	-28.0 0.49 (2)	10.00					
L-K	0 / 1908	-28.0	-28.0 0.42 (1)	10.00					
K-J	0 / 0	-28.0	-28.0 0.15 (3)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL (LL) = L/360 (1.13")
CALCULATED VERT. DEFL (LL) = L/999 (0.13")
ALLOWABLE DEFL (TL) = L/360 (1.13")
CALCULATED VERT. DEFL (TL) = L/999 (0.21")

CSI: TC=0.61 (E-F-1), BC=0.49 (N-P-2), WB=0.90 (E-N-1), SSI=0.28 (E-F-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

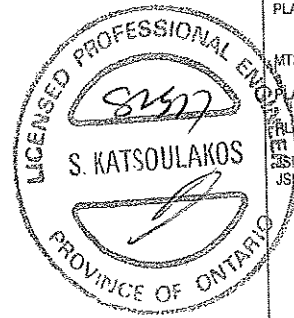
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

3S GRIP= 0.89 (B) (INPUT = 0.90)
JS METAL= 0.58 (M) (INPUT = 1.00)



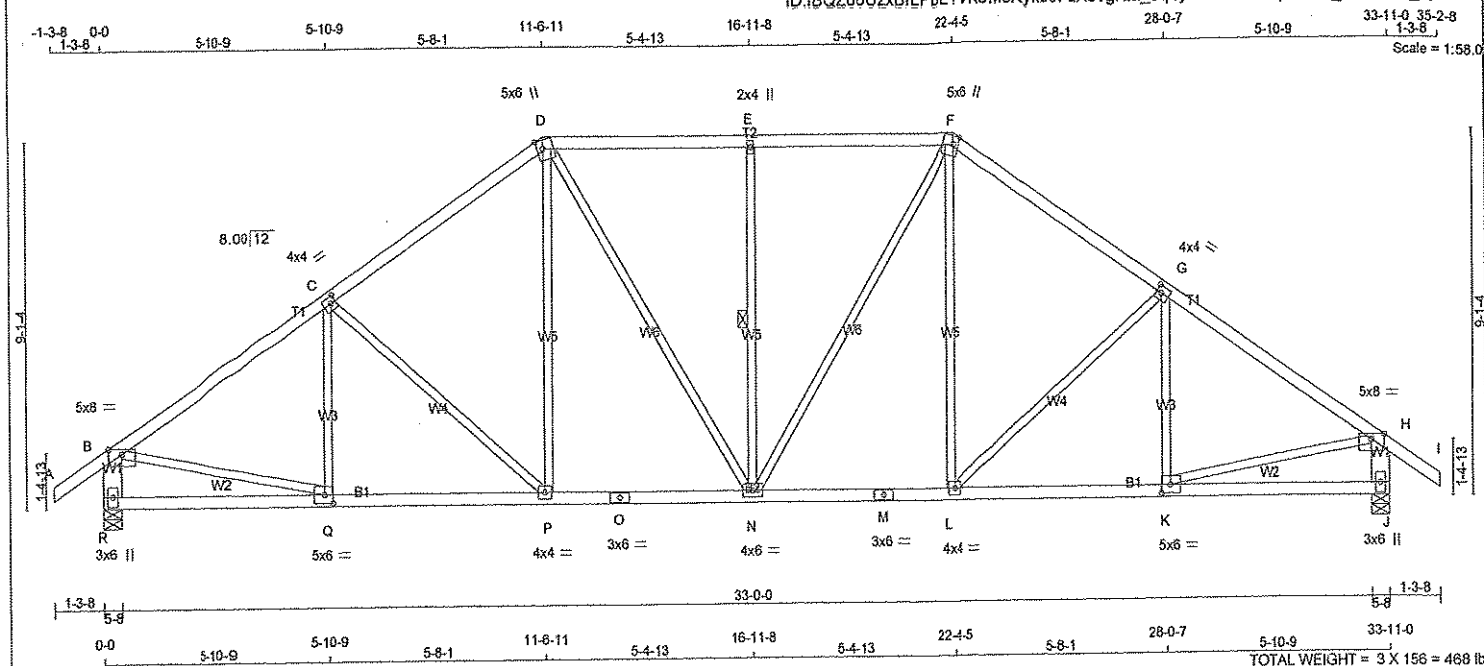
DWG NO. TAM 4326817
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
284265	T11	3	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID: BQZ66UzxBfLPbE7VK3moKykb9t-uXevgRM_3qfy7FIMAbOBqAizZl?_ZmOkPL_cyKKGv



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - I	2x4	DRY	No.2	SPF	
R - B	2x6	DRY	No.2	SPF	
J - H	2x6	DRY	No.2	SPF	
R - O	2x4	DRY	No.2	SPF	
O - M	2x4	DRY	No.2	SPF	
M - J	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	Edge	1.75
E	TMVW-w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	Edge	1.75
G	TMVW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW-p	MT20	5.0	8.0	Edge	
J	BMV1+p	MT20	3.0	6.0		
K	BMVW-t	MT20	5.0	6.0	2.50	2.75
L	BMVW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMVW-t	MT20	4.0	6.0		
O	BS-t	MT20	3.0	6.0		
P	BMVW-t	MT20	4.0	4.0		
Q	BMVW-t	MT20	5.0	6.0	2.50	2.75
R	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REORD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
R	2019	0	2019	0
J	2019	0	2019	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX	MIN	COMPONENT REACTIONS
	COMBINED	SNOW	LIVE	PERM. LIVE
R	1641	938 / 0	356 / 0	0 / 0
J	1641	938 / 0	356 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.08 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

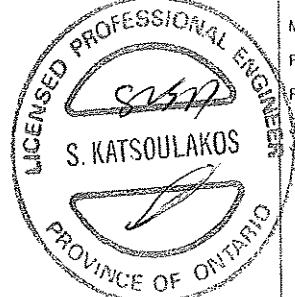
1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	WEBS
MEMB.	MEMB.
MAX. FACTORED	MAX. FACTORED
FORCE	FORCE
(LBS)	(LBS)
VERT. LOAD	VERT. LOAD
LC1	LC1
MAX	MAX
CSi (LC)	CSi (LC)
FR-TO	FR-TO
A-B	0 / 32
B-C	-2293 / 0
C-D	-2021 / 0
D-E	-1871 / 0
E-F	-1871 / 0
F-G	-2021 / 0
G-H	-2293 / 0
H-I	0 / 32
R-B	-1951 / 0
J-H	-1951 / 0
R-Q	0 / 0
Q-P	0 / 1934
P-O	0 / 1657
O-N	0 / 1657
N-M	0 / 1657
M-L	0 / 1657
L-K	0 / 1934
K-J	0 / 0



DWG NO. TAM 43269-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	PSF
DL	25.6	PSF
BOT CH.	LL	PSF
DL	10.5	PSF
DL	7.0	PSF
TOTAL LOAD	48.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.13")

CALCULATED VERT. DEFL. (LL) = L/999 (0.11")

ALLOWABLE DEFL. (TL) = L/360 (1.13")

CALCULATED VERT. DEFL. (TL) = L/999 (0.18")

CSI: TC=0.48 (B-C:1), BC=0.45 (P-Q:2), WB=0.44 (B-Q:1), SSI=0.22 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

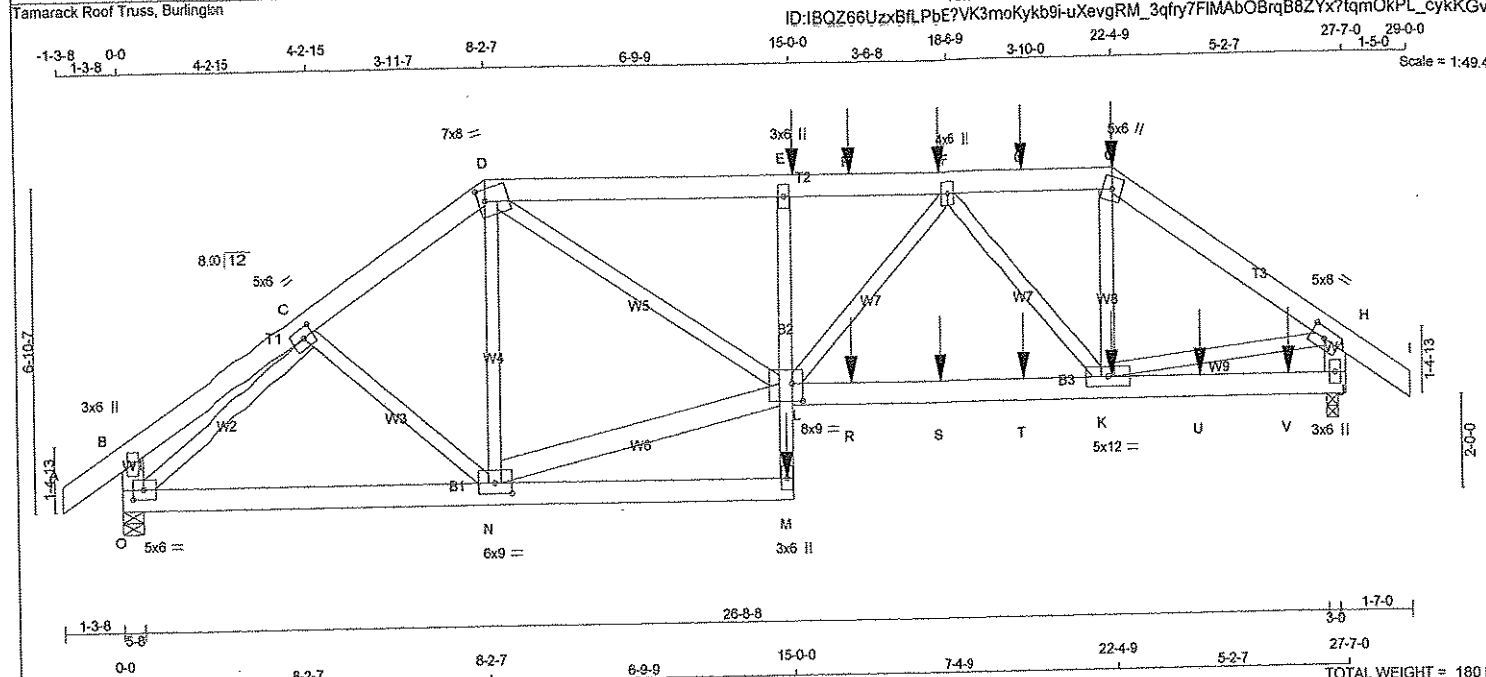
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.86 (Q) (INPUT = 0.90)

SI METAL= 0.48 (K) (INPUT = 1.00)



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x6	DRY	No.2
D - G	2x6	DRY	No.2
G - I	2x6	DRY	No.2
O - B	2x6	DRY	No.2
J - H	2x6	DRY	No.2
O - M	2x6	DRY	No.2
M - E	2x4	DRY	No.2
L - J	2x6	DRY	No.2
ALL WEBS	2x4	DRY	No.2
EXCEPT			
N - L	2x6	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	6.0		
C	TMVW+1	MT20	5.0	6.0	2.50	2.75
D	TTVW+m	MT20	7.0	8.0	3.00	1.75
E	TMV+p	MT20	3.0	6.0		
F	TMVW+1	MT20	4.0	6.0		
G	TTV+m	MT20	5.0	6.0		
H	TMVW+1	MT20	5.0	8.0	2.50	3.75
J	BMV+1+p	MT20	3.0	6.0		
K	BMVW+1	MT20	5.0	12.0		
L	BVMVW+1	MT20	8.0	9.0	4.50	2.75
M	BMV+p	MT20	3.0	6.0		
N	BMVW+1	MT20	6.0	9.0	2.75	4.50
O	BMVW+1	MT20	5.0	6.0	2.50	2.75

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 227.3 lbs FACTORED DOWN AT 22-4-9, 38.3 lbs FACTORED DOWN AT 15-0-12, 38.3 lbs FACTORED DOWN AT 16-3-13, AND 38.3 lbs FACTORED DOWN AT 20-3-13, AND 38.3 lbs FACTORED DOWN AT 20-3-13 ON TOP CHORD, AND 895.3 lbs FACTORED DOWN AT 14-10-4, 34.1 lbs FACTORED DOWN AT 16-3-13, 34.1 lbs FACTORED DOWN AT 18-3-13, 34.1 lbs FACTORED DOWN AT 20-3-13, 34.1 lbs FACTORED DOWN AT 22-3-13, AND 34.1 lbs FACTORED DOWN AT 24-3-13, AND 34.1 lbs FACTORED DOWN AT 26-3-13 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
J	2425	0	0	0
O	2114	0	0	0

UNFACTORED REACTIONS

1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	1971	1125 / 0	427 / 0	0 / 0	419 / 0	0 / 0
O	1716	984 / 0	369 / 0	0 / 0	362 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, O

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.97 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)	MEMB.	FORCE (LBS)	MAX FACTORED MAX (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 33	-84.3	-84.3 0.07 (1)	C-N	0 / 151	0.03 (3)	
B-C	-3 / 14	-84.3	-84.3 0.09 (1)	N-D	-353 / 49	0.18 (1)	
C-D	-2422 / 0	-84.3	-84.3 0.13 (1)	N-L	0 / 2062	0.27 (1)	
D-E	-3952 / 0	-84.3	-84.3 0.39 (1)	D-L	0 / 2343	0.41 (1)	
E-P	-3963 / 0	-84.3	-84.3 0.27 (1)	L-F	0 / 970	0.17 (1)	
P-F	-3963 / 0	-84.3	-84.3 0.27 (1)	F-K	-1408 / 0	0.55 (1)	
F-Q	-2453 / 0	-84.3	-84.3 0.15 (1)	K-G	0 / 987	0.17 (1)	
Q-G	-2453 / 0	-84.3	-84.3 0.15 (1)	O-C	-2847 / 0	0.87 (1)	
G-H	-2888 / 0	-84.3	-84.3 0.27 (1)	K-H	0 / 2430	0.43 (1)	
H-I	0 / 36	-84.3	-84.3 0.08 (1)				
O-B	-265 / 0	0.0	0.0 0.02 (1)				
J-H	-2353 / 0	0.0	0.0 0.17 (1)				
O-N	0 / 1981	-28.0	-28.0 0.43 (2)				
N-M	0 / 23	-28.0	-28.0 0.25 (3)				
M-L	0 / 760	0.0	0.0 0.20 (1)				
L-E	-566 / 0	0.0	0.0 0.13 (1)				
L-R	0 / 3341	-28.0	-28.0 0.57 (1)				
R-S	0 / 3341	-28.0	-28.0 0.57 (1)				
S-T	0 / 3341	-28.0	-28.0 0.57 (1)				
T-K	0 / 3341	-28.0	-28.0 0.57 (1)				
K-U	0 / 0	-28.0	-28.0 0.24 (3)				
U-V	0 / 0	-28.0	-28.0 0.24 (3)				
V-J	0 / 0	-28.0	-28.0 0.24 (3)				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TOTAL
E	15-0-12	-38	-38	FRONT	VERT	TOTAL
F	18-3-13	-38	-38	FRONT	VERT	TOTAL
G	22-4-9	-18	-20	FRONT	VERT	TOTAL
G	22-4-9	-42	-42	FRONT	VERT	TOTAL
G	22-4-9	-168	-168	FRONT	VERT	TOTAL
K	22-3-13	-20	-34	FRONT	VERT	TOTAL
M	14-10-4	-695	-695	FRONT	VERT	TOTAL
P	16-3-13	-38	-38	FRONT	VERT	TOTAL
Q	20-3-13	-20	-34	FRONT	VERT	TOTAL
R	16-3-13	-20	-34	FRONT	VERT	TOTAL
S	18-3-13	-20	-34	FRONT	VERT	TOTAL
T	20-3-13	-20	-34	FRONT	VERT	TOTAL
U	24-3-13	-20	-34	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.92")
CALCULATED VERT. DEFL.(LL) = L/999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.92")
CALCULATED VERT. DEFL.(TL) = L/999 (0.22")

CSI: TC=0.39 (D-E:1), BC=0.57 (K-L:1), WB=0.87 (C-O:1), SSI=0.21 (D-E:1)

COL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

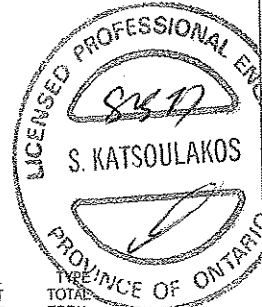
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.90 (D) (INPUT = 0.90)
JSI METAL=0.89 (C) (INPUT = 1.00)



DWG NO. TAM
STRUCTURAL
COMPONENT ONLY

JOB NAME 284265	TRUSS NAME T12	QUANTITY 1	PLY 1	JOB DESC. 43954 TRUSS DESC.	DRWG NO.
Version 8.030 S Oct 5 2016 Mitek Industries, Inc. Fri Aug 25 12:03:16 2017 Page 2 ID:1BQZ66Uzx8fLPbE?VK3moKykb9i-uXevgRM 3qfry7FIMAbOBqB8ZYx?tmOkPL_cykKGv					

Tamarack Roof Truss, Burlington

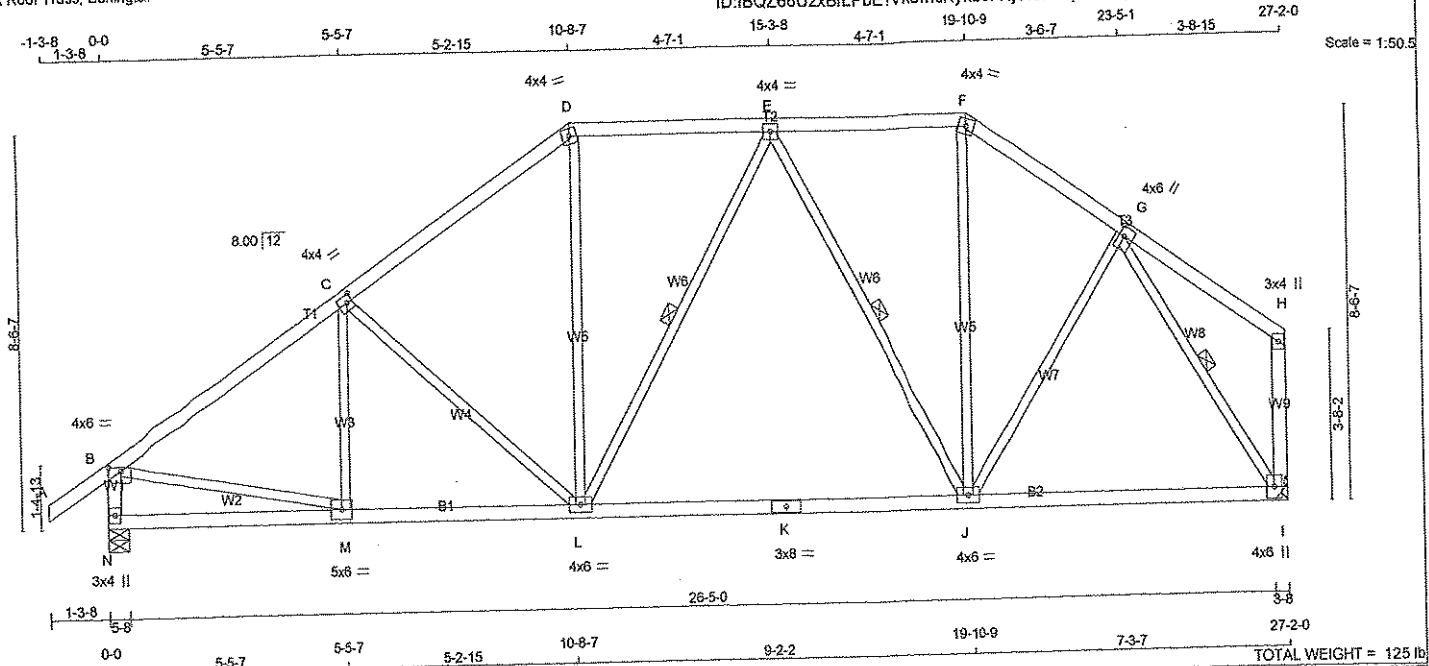
FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
V	28-3-13	-20	-34	--	FRONT	VERT	TOTAL



DWG NO. TAM 4320-17
STRUCTURAL
COMPONENT ONLY

Per



LUMBER

N. L. G. A. RULES

CHORDS

SIZE

LUMBER

DESCR.

A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	6.0	1.00	3.25
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMVW-t	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	4.0		
G	TMVW-t	MT20	4.0	6.0	2.50	1.50
H	TMV-p	MT20	3.0	4.0		
I	BMVW1+p	MT20	4.0	6.0		
J	BMVWVW-t	MT20	4.0	6.0		
K	BS-t	MT20	3.0	8.0		
L	BMVWVW-t	MT20	4.0	6.0		
M	BMVWVW-t	MT20	5.0	6.0		
N	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	UP
JT	1640	0	1640	0
I	1525	0	1525	0

MIN. SEAT SIZE: 3-8

UNFACTORED REACTIONS

	1ST LCASE	MAX /MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE	
JT	1330	765 / 0	285 / 0	0 / 0	280 / 0
I	1252	695 / 0	285 / 0	0 / 0	272 / 0

BEARING MATERIAL TO BE SPF NO 2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.68 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-L, E-J, G-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		WEBS		MAX. FACTORED	
	FORCE (LBS)	VERT. LOAD (PLF)	CS1 (LC)	UNBRAC LENGTH	MEMB.	FORCE (LBS)	CS1 (LC)	UNBRAC LENGTH
FR-TO					FR-TO			
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	M-C	-148 / 115	0.06 (1)
B-C	-1751 / 0	-84.3	-84.3	0.37 (1)	4.68	C-L	-356 / 0	0.35 (1)
C-D	-1492 / 0	-84.3	-84.3	0.34 (1)	5.00	L-D	0 / 495	0.11 (1)
D-E	-1220 / 0	-84.3	-84.3	0.24 (1)	5.54	E-J	-57 / 75	0.04 (1)
E-F	-1019 / 0	-84.3	-84.3	0.23 (1)	5.93	J-F	-479 / 0	0.30 (1)
F-G	-1239 / 0	-84.3	-84.3	0.15 (1)	5.62	J-G	0 / 408	0.09 (1)
G-H	0 / 19	-84.3	-84.3	0.17 (1)	10.00	J-G	0 / 329	0.07 (2)
H-B	-1574 / 0	0.0	0.0	0.16 (1)	6.58	B-M	0 / 1510	0.34 (1)
I-H	-119 / 0	0.0	0.0	0.03 (1)	7.81	G-I	-1569 / 0	0.49 (1)
N-M	0 / 0	-28.0	-28.0	0.18 (2)	10.00			
M-L	0 / 1481	-28.0	-28.0	0.50 (2)	10.00			
L-K	0 / 1248	-28.0	-28.0	0.62 (2)	10.00			
K-J	0 / 1248	-28.0	-28.0	0.62 (2)	10.00			
J-I	0 / 834	-28.0	-28.0	0.57 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
	DL = 3.0 PSF
BOT CH.	LL = 10.5 PSF
	DL = 7.0 PSF
TOTAL LOAD	= 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014

- CSA 086-09

- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.91")

CALCULATED VERT. DEFL.(LL) = L/999 (0.16")

ALLOWABLE DEFL.(TL) = L/360 (0.91")

CALCULATED VERT. DEFL.(TL) = L/999 (0.26")

CSI: TC=0.37 (B-C:1), BC=0.62 (J-L:2), WB=0.49 (G-I:1), SS=0.19 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

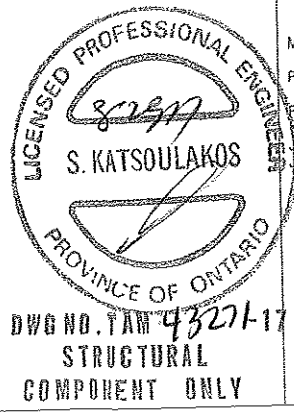
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MT20	618	354	1687 822 2284 1656

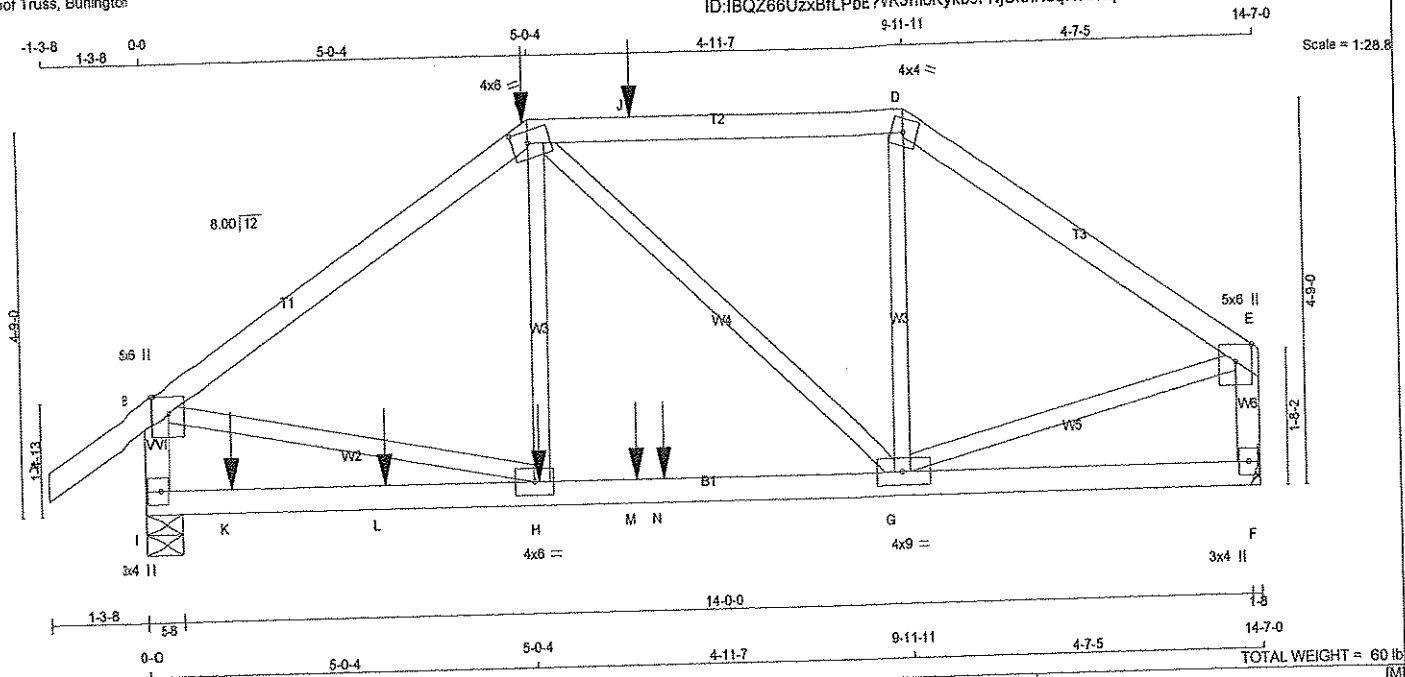
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

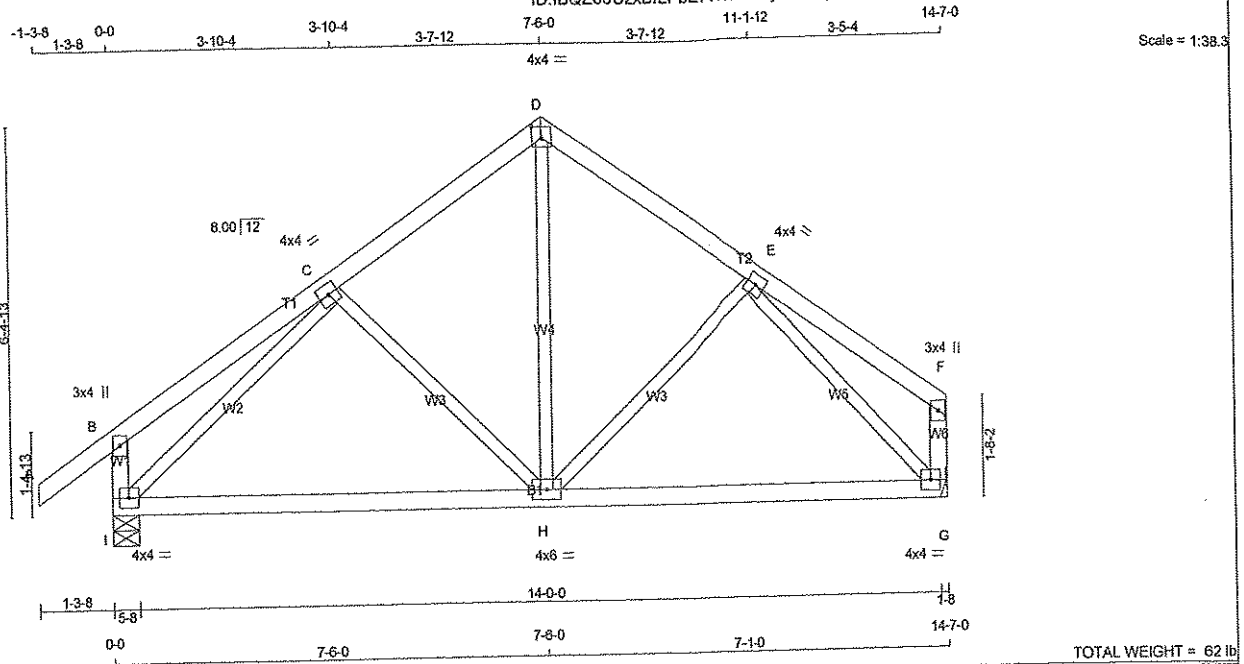
SI GRIP= 0.87 (G) (INPUT = 0.90)

SI METAL= 0.87 (K) (INPUT = 1.00)





LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
G - H	2x4	DRY	No.2
H - I	2x4	DRY	No.2
I - J	2x4	DRY	No.2
J - K	2x4	DRY	No.2
K - L	2x4	DRY	No.2
L - M	2x4	DRY	No.2
M - N	2x4	DRY	No.2
N - O	2x4	DRY	No.2
O - P	2x4	DRY	No.2
P - Q	2x4	DRY	No.2
Q - R	2x4	DRY	No.2
R - S	2x4	DRY	No.2
S - T	2x4	DRY	No.2
T - U	2x4	DRY	No.2
U - V	2x4	DRY	No.2
V - W	2x4	DRY	No.2
W - X	2x4	DRY	No.2
X - Y	2x4	DRY	No.2
Y - Z	2x4	DRY	No.2
Z - AA	2x4	DRY	No.2
AA - AB	2x4	DRY	No.2
AB - AC	2x4	DRY	No.2
AC - AD	2x4	DRY	No.2
AD - AE	2x4	DRY	No.2
AE - AF	2x4	DRY	No.2
AF - AG	2x4	DRY	No.2
AG - AH	2x4	DRY	No.2
AH - AI	2x4	DRY	No.2
AI - AJ	2x4	DRY	No.2
AJ - AK	2x4	DRY	No.2
AK - AL	2x4	DRY	No.2
AL - AM	2x4	DRY	No.2
AM - AN	2x4	DRY	No.2
AN - AO	2x4	DRY	No.2
AO - AP	2x4	DRY	No.2
AP - AQ	2x4	DRY	No.2
AQ - AR	2x4	DRY	No.2
AR - AS	2x4	DRY	No.2
AS - AT	2x4	DRY	No.2
AT - AU	2x4	DRY	No.2
AU - AV	2x4	DRY	No.2
AV - AW	2x4	DRY	No.2
AW - AX	2x4	DRY	No.2
AX - AY	2x4	DRY	No.2
AY - AZ	2x4	DRY	No.2
AZ - BA	2x4	DRY	No.2
BA - BB	2x4	DRY	No.2
BB - BC	2x4	DRY	No.2
BC - BD	2x4	DRY	No.2
BD - BE	2x4	DRY	No.2
BE - BF	2x4	DRY	No.2
BF - BG	2x4	DRY	No.2
BG - BH	2x4	DRY	No.2
BH - BI	2x4	DRY	No.2
BI - BJ	2x4	DRY	No.2
BJ - BK	2x4	DRY	No.2
BK - BL	2x4	DRY	No.2
BL - BM	2x4	DRY	No.2
BM - BN	2x4	DRY	No.2
BN - BO	2x4	DRY	No.2
BO - BP	2x4	DRY	No.2
BP - BQ	2x4	DRY	No.2
BQ - BR	2x4	DRY	No.2
BR - BS	2x4	DRY	No.2
BS - BT	2x4	DRY	No.2
BT - BU	2x4	DRY	No.2
BU - BV	2x4	DRY	No.2
BV - BW	2x4	DRY	No.2
BW - BX	2x4	DRY	No.2
BX - BY	2x4	DRY	No.2
BY - BZ	2x4	DRY	No.2
BZ - CA	2x4	DRY	No.2
CA - CB	2x4	DRY	No.2
CB - CC	2x4	DRY	No.2
CC - CD	2x4	DRY	No.2
CD - CE	2x4	DRY	No.2
CE - CF	2x4	DRY	No.2
CF - CG	2x4	DRY	No.2
CG - CH	2x4	DRY	No.2
CH - CI	2x4	DRY	No.2
CI - CJ	2x4	DRY	No.2
CJ - CK	2x4	DRY	No.2
CK - CL	2x4	DRY	No.2
CL - CM	2x4	DRY	No.2
CM - CN	2x4	DRY	No.2
CN - CO	2x4	DRY	No.2
CO - CP	2x4	DRY	No.2
CP - CQ	2x4	DRY	No.2
CQ - CR	2x4	DRY	No.2
CR - CS	2x4	DRY	No.2
CS - CT	2x4	DRY	No.2
CT - CU	2x4	DRY	No.2
CU - CV	2x4	DRY	No.2
CV - CW	2x4	DRY	No.2
CW - CX	2x4	DRY	No.2
CX - CY	2x4	DRY	No.2
CY - CZ	2x4	DRY	No.2
CZ - DA	2x4	DRY	No.2
DA - DB	2x4	DRY	No.2
DB - DC	2x4	DRY	No.2
DC - DD	2x4	DRY	No.2
DD - DE	2x4	DRY	No.2
DE - DF	2x4	DRY	No.2
DF - DG	2x4	DRY	No.2
DG - DH	2x4	DRY	No.2
DH - DI	2x4	DRY	No.2
DI - DJ	2x4	DRY	No.2
DJ - DK	2x4	DRY	No.2
DK - DL	2x4	DRY	No.2
DL - DM	2x4	DRY	No.2
DM - DN	2x4	DRY	No.2
DN - DO	2x4	DRY	No.2
DO - DP	2x4	DRY	No.2
DP - DQ	2x4	DRY	No.2
DQ - DR	2x4	DRY	No.2
DR - DS	2x4	DRY	No.2
DS - DT	2x4	DRY	No.2
DT - DU	2x4	DRY	No.2
DU - DV	2x4	DRY	No.2
DV - DW	2x4	DRY	No.2
DW - DX	2x4	DRY	No.2
DX - DY	2x4	DRY	No.2
DY - DZ	2x4	DRY	No.2
DZ - EA	2x4	DRY	No.2
EA - EB	2x4	DRY	No.2
EB - EC	2x4	DRY	No.2
EC - ED	2x4	DRY	No.2
ED - EE	2x4	DRY	No.2
EE - EF	2x4	DRY	No.2
EF - EG	2x4	DRY	No.2
EG - EH	2x4	DRY	No.2
EH - EI	2x4	DRY	No.2
EI - EJ	2x4	DRY	No.2
EJ - EK	2x4	DRY	No.2
EK - EL	2x4	DRY	No.2
EL - EM	2x4	DRY	No.2
EM - EN	2x4	DRY	No.2
EN - EO	2x4	DRY	No.2
EO - EP	2x4	DRY	No.2
EP - EQ	2x4	DRY	No.2
EQ - ER	2x4	DRY	No.2
ER - ES	2x4	DRY	No.2
ES - ET	2x4	DRY	No.2
ET - EU	2x4	DRY	No.2
EU - EV	2x4	DRY	No.2
EV - EW	2x4	DRY	No.2
EW - EX	2x4	DRY	No.2
EX - EY	2x4	DRY	No.2
EY - EZ	2x4	DRY	No.2
EZ - FA	2x4	DRY	No.2
FA - FB	2x4	DRY	No.2
FB - FC	2x4	DRY	No.2
FC - FD	2x4	DRY	No.2
FD - FE	2x4	DRY	No.2
FE - FF	2x4	DRY	No.2
FF - FG	2x4	DRY	No.2
FG - FH	2x4	DRY	No.2
FH - FI	2x4	DRY	No.2
FI - FJ	2x4	DRY	No.2
FJ - FK	2x4	DRY	No.2
FK - FL	2x4	DRY	No.2
FL - FM	2x4	DRY	No.2
FM - FN	2x4	DRY	No.2
FN - FO	2x4	DRY	No.2
FO - FP	2x4	DRY	No.2
FP - FQ	2x4	DRY	No.2
FQ - FR	2x4	DRY	No.2
FR - FS	2x4	DRY	No.2
FS - FT	2x4	DRY	No.2
FT - FU	2x4	DRY	No.2
FU - FV	2x4	DRY	No.2
FV - FW	2x4	DRY	No.2
FW - FX	2x4	DRY	No.2
FX - FY	2x4	DRY	No.2
FY - FZ	2x4	DRY	No.2
FZ - GA	2x4	DRY	No.2
GA - GB	2x4	DRY	No.2
GB - GC	2x4	DRY	No.2
GC - GD	2x4	DRY	No.2
GD - GE	2x4	DRY	No.2
GE - GF	2x4	DRY	No.2
GF - GG	2x4	DRY	No.2
GG - GH	2x4	DRY	No.2
GH - GI	2x4	DRY	No.2
GI - GJ	2x4	DRY	No.2
GJ - GK	2x4	DRY	No.2
GK - GL	2x4	DRY	No.2
GL - GM	2x4	DRY	No.2
GM - GN	2x4	DRY	No.2
GN - GO	2x4	DRY	No.2
GO - GP	2x4	DRY	No.2
GP - GQ	2x4	DRY	No.2
GQ - GR	2x4	DRY	No.2
GR - GS	2x4	DRY	No.2
GS - GT	2x4	DRY	No.2
GT - GU	2x4	DRY	No.2
GU - GV	2x4	DRY	No.2
GV - GW	2x4	DRY	No.2
GW - GX	2x4	DRY	No.2
GX - GY	2x4	DRY	No.2
GY - GZ	2x4	DRY	No.2
GZ - HA	2x4	DRY	No.2
HA - HB	2x4	DRY	No.2
HB - HC	2x4	DRY	No.2
HC - HD	2x4	DRY	No.2
HD - HE	2x4	DRY	No.2
HE - HF	2x4	DRY	No.2
HF - HG	2x4	DRY	No.2
HG - HH	2x4	DRY	No.2
HH - HI	2x4	DRY	No.2
HI - HJ	2x4	DRY	No.2
HJ - HK	2x4	DRY	No.2
HK - HL	2x4	DRY	No.2
HL - HM	2x4	DRY	No.2
HM - HN	2x4	DRY	No.2
HN - HO	2x4	DRY	No.2
HO - HP	2x4	DRY	No.2
HP - HQ	2x4	DRY	No.2
HQ - HR	2x4	DRY	No.2
HR - HS	2x4	DRY	No.2
HS - HT	2x4	DRY	No.2
HT - HU	2x4	DRY	No.2
HU - HV	2x4	DRY	No.2
HV - HW	2x4	DRY	No.2
HW - HX	2x4	DRY	No.2
HX - HY	2x4	DRY	No.2
HY - HZ	2x4	DRY	No.2
HZ - IA	2x4	DRY	No.2
IA - IB	2x4	DRY	No.2
IB - IC	2x4	DRY	No.2
IC - ID	2x4	DRY	No.2
ID - IE	2x4	DRY	No.2
IE - IF	2x4	DRY	No.2
IF - IG	2x4	DRY	No.2
IG - IH	2x4	DRY	No.2
IH - II	2x4	DRY	No.2
II - IJ	2x4	DRY	No.2
IJ - IK	2x4	DRY	No.2
IK - IL	2x4	DRY	No.2
IL - IM	2x4	DRY	No.2
IM - IN	2x4	DRY	No.2
IN - IO	2x4	DRY	No.2
IO - IP	2x4	DRY	No.2
IP - IQ	2x4	DRY	No.2
IQ - IR	2x4	DRY	No.2
IR - IS	2x4	DRY	No.2
IS - IT	2x4	DRY	No.2
IT - IU	2x4	DRY	No.2
IU - IV	2x4	DRY	No.2
IV - IW	2x4	DRY	No.2
IW - IX	2x4	DRY	No.2
IX - IY	2x4	DRY	No.2
IY - IZ	2x4	DRY	No.2
IZ - JA	2x4	DRY	No.2
JA - JB	2x4	DRY	No.2
JB - JC	2x4	DRY	No.2
JC - JD	2x4	DRY	No.2
JD - JE	2x4	DRY	No.2
JE - JF	2x4	DRY	No.2
JF - JG	2x4	DRY	No.2
JG - JH	2x4	DRY	No.2
JH - JI	2x4	DRY	No.2
JI - JJ	2x4	DRY	No.2
JJ - JK	2x4	DRY	No.2
JK - JL	2x4	DRY	No.2
JL - JM	2x4	DRY	No.2
JM - JN	2x4	DRY	No.2
JN - JO	2x4	DRY	No.2
JO - JP	2x4	DRY	No.2
JP - JQ	2x4	DRY	No.2
JQ - JR	2x4	DRY	No.2
JR - JS	2x4	DRY	No.2
JS - JT	2x4	DRY	No.2
JT - JU	2x4	DRY	No.2
JU - JV	2x4	DRY	No.2
JV - JW	2x4	DRY	No.2
JW - JX	2x4	DRY	No.2
JX - JY	2x4	DRY	No.2
JY - JZ	2x4	DRY	No.2
JZ - KA	2x4	DRY	No.2
KA - KB	2x4	DRY	No.2
KB - KC	2x4	DRY	No.2
KC - KD	2x4	DRY	No.2
KD - KE	2x4	DRY	No.2
KE - KF	2x4	DRY	No.2
KF - KG	2x4	DRY	No.2
KG - KH	2x4	DRY	No.2
KH - KI	2x4	DRY	No.2
KI - KJ	2x4	DRY	No.2
KJ - KK	2x4	DRY	No.2
KK - KL	2x4	DRY	No.2
KL - KM	2x4	DRY	No.2
KM - KN	2x4	DRY	No.2
KN - KO	2x4	DRY	No.2
KO - KP	2x4	DRY	No.2
KP - KQ	2x4	DRY	No.2
KQ - KR	2x4	DRY	No.2
KR - KS	2x4	DRY	No.2
KS - KT	2x4	DRY	No.2
KT - KU	2x4	DRY	No.2
KU - KV	2x4	DRY	No.2
KV - KW	2x4	DRY	No.2
KW - KX	2x4	DRY	No.2
KX - KY	2x4	DRY	No.2
KY - KZ	2x4	DRY	No.2
KZ - LA	2x4	DRY	No.2
LA - LB	2x4	DRY	No.2
LB - LC	2x4	DRY	No.2
LC - LD	2x4	DRY	No.2
LD - LE	2x4	DRY	No.2
LE - LF	2x4	DRY	No.2
LF - LG	2x4	DRY	No.2
LG - LH	2x4	DRY	No.2
LH - LI	2x4	DRY	No.2
LI - LJ	2x4	DRY	No.2
LJ - LK	2x4	DRY	No.2
LK - LL	2x4	DRY	No.2
LL - LM	2x4	DRY	No.2
LM - LN	2x4	DRY	No.2
LN - LO	2x4	DRY	No.2
LO - LP	2x4	DRY	No.2
LP - LQ	2x4	DRY	No.2
LQ - LR	2x4	DRY	No.2
LR - LS	2x4	DRY	No.2
LS - LT	2x4	DRY	No.2
LT - LU	2x4	DRY	No.2
LU - LV	2x4	DRY	No.2
LV - LW	2x4	DRY	No.2
LW - LX	2x4	DRY	No.2
LX - LY	2x4	DRY	No.2
LY - LZ	2x4	DRY	No.2
LZ - MA	2x4	DRY	No.2
MA - MB	2x4	DRY	No.2
MB - MC	2x4	DRY	No.2
MC - MD	2x4	DRY	No.2
MD - ME	2x4	DRY	No.2
ME - MF	2x4	DRY	No.2
MF - MG	2x4	DRY	No.2
MG - MH	2x4	DRY	No.2
MH - MI	2x4	DRY	No.2
MI - MJ	2x4	DRY	No.2
MJ - MK	2x4	DRY	No.2
MK - ML	2x4	DRY	No.2
ML - MM	2x4	DRY	No.2
MM - MN	2x4	DRY	No.2
MN - MO	2x4	DRY	No.2
MO - MP	2x4	DRY	No.2
MP - MQ	2x4	DRY	No.2
MQ - MR	2x4	DRY	No.2
MR - MS	2x4	DRY	No.2
MS - MT	2x4	DRY	No.2
MT - MU	2x4	DRY	No.2
MU - MV	2x4	DRY	No.2
MV - MW	2x4	DRY	No.2
MW - MX	2x4	DRY	No.2
MX - MY	2x4	DRY	No.2
MY - MZ	2x4	DRY	



TOTAL WEIGHT = 62 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
I - B	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
I - G	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF EXCEPT

DRY: SEASONED LUMBER

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0			
C	TMWW-t	MT20	4.0	4.0			
D	TTW-p	MT20	4.0	4.0	2.25	2.00	
E	TMWW-t	MT20	4.0	4.0			
F	TMV+p	MT20	3.6	4.0			
G	BMVW1-t	MT20	4.0	4.0			
H	BMVW1-t	MT20	4.0	6.0			
I	BMVW1-t	MT20	4.0	4.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	GROSS REACTION	MAXIMUM FACTORED	INPUT	REQRD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
I	934	0	934	0	0
G	819	0	819	0	0

UNFACTORED REACTIONS	1ST LCASE	MAX /MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
I	751	443 / 0	153 / 0
G	672	373 / 0	153 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX.	MEMB.	MAX. FACTORED	MAX
MEMB.	FORCE	VERT. LOAD	LC1	MAX	MAX.	MEMB.	FORCE	MAX	
FR-TO	FROM	TO	LENGTH	FR-TO	FROM	TO	LENGTH	FR-TO	FROM
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	C-H	-172 / 40	0.08 (1)	
B-C	0 / 20	-84.3	-84.3	0.19 (1)	10.00	H-D	0 / 449	0.10 (2)	
C-D	-845 / 0	-84.3	-84.3	0.15 (1)	6.25	H-E	-105 / 63	0.05 (1)	
D-E	-842 / 0	-84.3	-84.3	0.13 (1)	6.25	I-C	-891 / 0	0.38 (1)	
E-F	0 / 20	-84.3	-84.3	0.17 (1)	10.00	E-G	-875 / 0	0.33 (1)	
I-B	-237 / 0	0.0	0.0	0.02 (1)	7.81				
G-F	-104 / 0	0.0	0.0	0.01 (1)	7.81				
I-H	0 / 644	-28.0	-28.0	0.50 (2)	10.00				
H-G	0 / 598	-28.0	-28.0	0.50 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.49")
CALCULATED VERT. DEFL.(LL) = L/999 (0.10")
ALLOWABLE DEFL.(TL) = L/360 (0.49")
CALCULATED VERT. DEFL.(TL) = L/999 (0.16")

CSI: TC=0.19 (B-C:1), BC=0.50 (H-I:2), WB=0.38 (C-I:1), SSI=0.18 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (E) (INPUT = 0.80)
JSI METAL= 0.31 (C) (INPUT = 1.00)



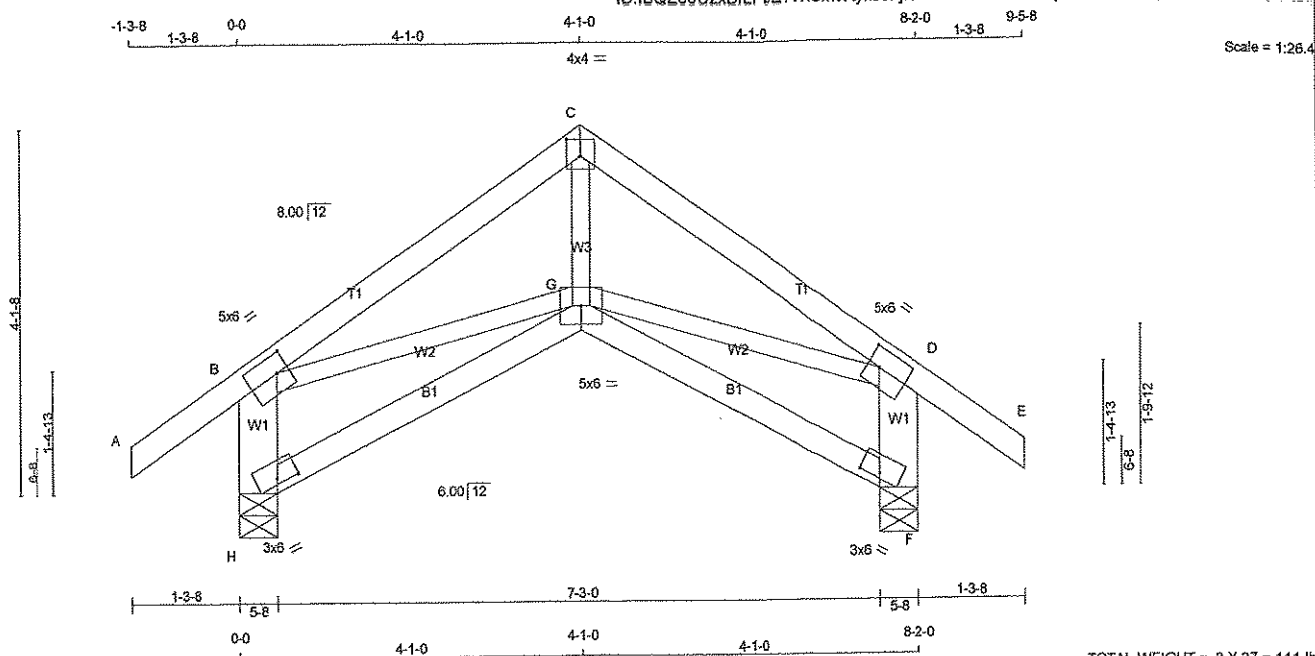
DWG NO. TAM 43273-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
284266	T16	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 3 X 37 = 111 lb

LUMBER					
N. L. G. A. RULES					
CHORDS		SIZE	LUMBER		DESCR
A - C	2x4	DRY	No.2		SPF
C - E	2x4	DRY	No.2		SPF
H - B	2x6	DRY	No.2		SPF
F - D	2x6	DRY	No.2		SPF
H - G	2x4	DRY	No.2		SPF
G - F	2x4	DRY	No.2		SPF
ALL WEBS EXCEPT		2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	1.75
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMVW-t	MT20	5.0	6.0	2.50	1.75
F	BVM1-I	MT20	3.0	6.0	0.75	3.00
G	BBWWW-p	MT20	5.0	6.0		
H	BVM1-I	MT20	3.0	6.0	0.75	3.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	HORZ	GROSS REACTION	GROSS REACTION	DOWN	HORZ	UPLIFT	IN-SX	BRG
H	574	0	574	0	0	0	5-8	5-8	5-8
F	574	0	574	0	0	0	5-8	5-8	5-8

UNFACTORED REACTIONS		1ST LCASE		MAX/MIN COMPONENT REACTIONS		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	JT	COMBINED	SNOW	LIVE
H	455	279 / 0	88 / 0	0 / 0	0 / 0	90 / 0	0 / 0	H	455	279 / 0	88 / 0
F	455	279 / 0	88 / 0	0 / 0	0 / 0	90 / 0	0 / 0	F	455	279 / 0	88 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING		TOTAL LOAD CASES: (4)		CHORDS		FACTORED		MEMB.		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT.	LOAD (PLF)	LC1	MAX	MEMB.	FORCE (LBS)	MAX	MEMB.	FORCE (LBS)	MAX	MEMB.	FORCE (LBS)
FR-TO						FR-TO			FR-TO			FR-TO	
A-B	0 / 32	-84.3	-84.3	0.11 (1)	10.00	A-B	0 / 32	0.07 (2)	A-B	0 / 32	0.07 (2)	A-B	0 / 32
B-C	-570 / 0	-84.3	-84.3	0.18 (1)	6.25	B-C	-570 / 0	0.11 (1)	B-C	-570 / 0	0.11 (1)	B-C	-570 / 0
C-D	-570 / 0	-84.3	-84.3	0.18 (1)	6.25	C-D	-570 / 0	0.11 (1)	C-D	-570 / 0	0.11 (1)	C-D	-570 / 0
D-E	0 / 32	-84.3	-84.3	0.11 (1)	10.00	D-E	0 / 32	0.07 (2)	D-E	0 / 32	0.07 (2)	D-E	0 / 32
H-B	-517 / 0	0.0	0.0	0.04 (1)	7.81	H-B	-517 / 0	0.04 (1)	H-B	-517 / 0	0.04 (1)	H-B	-517 / 0
F-D	-517 / 0	0.0	0.0	0.04 (1)	7.81	F-D	-517 / 0	0.04 (1)	F-D	-517 / 0	0.04 (1)	F-D	-517 / 0
H-G	0 / 0	-28.0	-28.0	0.14 (3)	10.00	H-G	0 / 0	0.14 (3)	H-G	0 / 0	0.14 (3)	H-G	0 / 0
G-F	0 / 0	-28.0	-28.0	0.14 (3)	10.00	G-F	0 / 0	0.14 (3)	G-F	0 / 0	0.14 (3)	G-F	0 / 0

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.27")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.03")
ALLOWABLE DEFL.(TL) = $L/360$ (0.27")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.04")

CSI: TC=0.18 (C-D:1), BC=0.14 (F-G:3), WB=0.11 (B-G:1), SSI=0.11 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1658

PLATE PLACEMENT TOL. = 0.250 inches

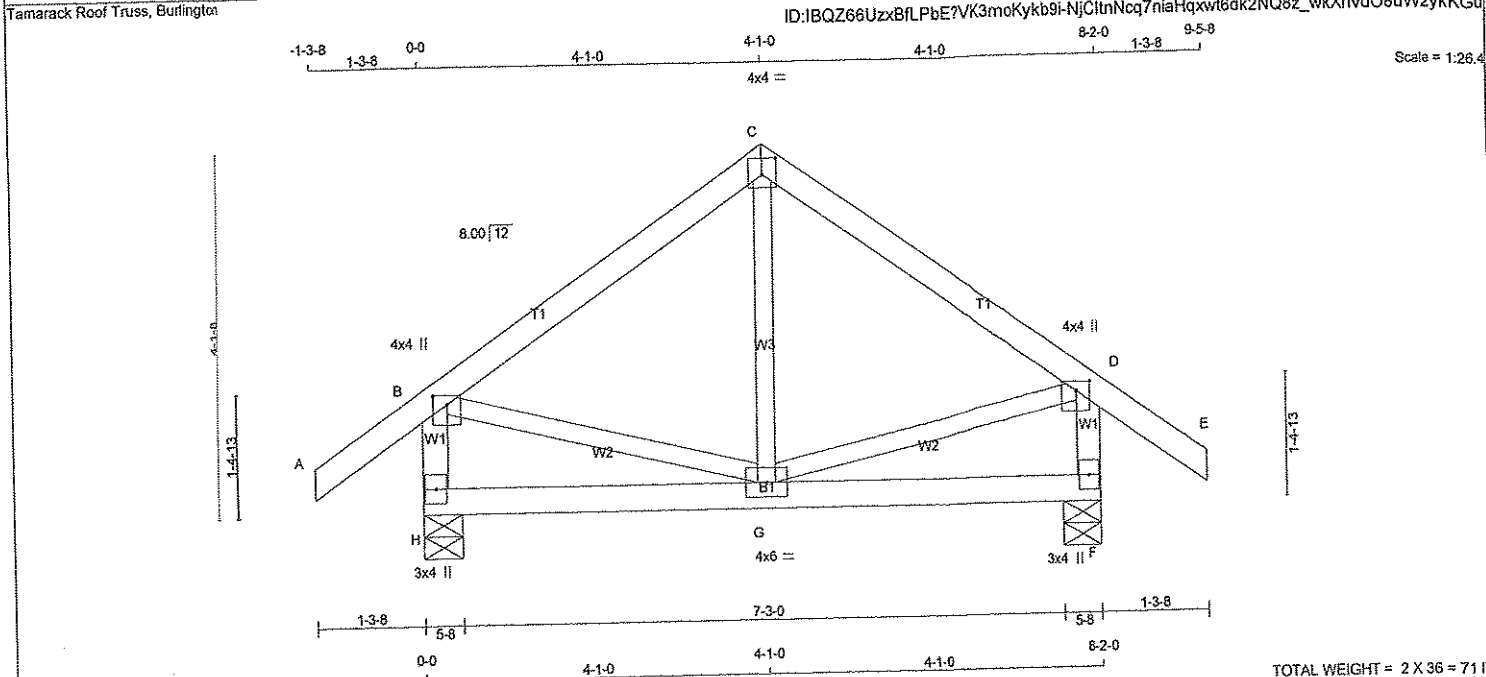
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.55 (C) (INPUT = 0.90)
JSI METAL= 0.18 (B) (INPUT = 1.00)



DWG NO. TAM4328-17

STRUCTURAL
COMPONENT ONLY



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4 DRY	No.2	SPF
C - E	2x4 DRY	No.2	SPF
H - B	2x4 DRY	No.2	SPF
F - D	2x4 DRY	No.2	SPF
H - F	2x4 DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMVW+p	MT20	4.0	4.0 1.25 2.00
C	TTW-p	MT20	4.0	4.0 2.25 2.00
D	TMVW+p	MT20	4.0	4.0 1.25 2.00
F	BMV1+p	MT20	3.0	4.0
G	BMVWW-1	MT20	4.0	6.0
H	BMV1+p	MT20	3.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
H	574	0	574	0	0
F	574	0	574	0	0

UNFACTORED REACTIONS		1ST LCASE	MAX/MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE	PERM. LIVE
H	455	279 / 0	86 / 0	0 / 0
F	455	279 / 0	86 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. MEMB. CS (LC)	MAX. UNBRACED LENGTH
FR-TO	FROM TO				
A-B	0 / 32	-34.3	-84.3	0.11 (1)	10.00
B-C	-313 / 0	-84.3	-84.3	0.18 (1)	6.25
C-D	-313 / 0	-84.3	-84.3	0.18 (1)	6.25
D-E	0 / 32	-84.3	-84.3	0.11 (1)	10.00
H-B	-530 / 0	0.0	0.0	0.05 (1)	7.81
F-D	-530 / 0	0.0	0.0	0.05 (1)	7.81
H-G	0 / 0	-28.0	-28.0	0.14 (3)	10.00
G-F	0 / 0	-28.0	-28.0	0.14 (3)	10.00



DWG NO. TAM 43274-17
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.27")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.01")
ALLOWABLE DEFL.(TL) = $L/360$ (0.27")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.02")

CSI: TC=0.18 (C-D:1), BC=0.14 (F-G:3), WB=0.08 (B-G:1), SSI=0.11 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

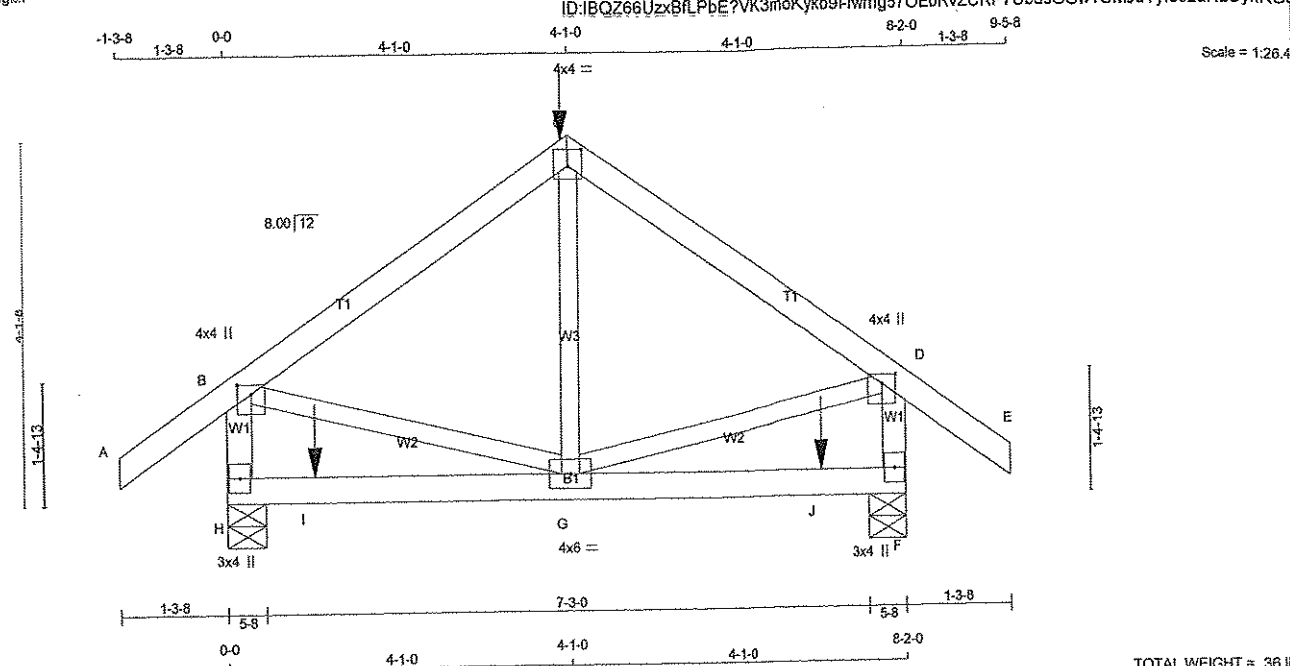
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1655

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.39 (G) (INPUT = 0.90)
JSI METAL= 0.11 (D) (INPUT = 1.00)



LUMBER

N. L. G. A. RULES

CHORDS SIZE

LUMBER

DESCR.

A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

DRY: SEASONED LUMBER

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMVW+p	MT20	4.0	4.0	1.25	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMVWW-I	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

HANGERS NOTES

1) SPECIAL HANGER(S)OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 172.0 lbs FACTORED DOWN AT 4-1-0 ON TOP CHORD, AND 139.2 lbs FACTORED DOWN AT 1-0-8, AND 139.2 lbs FACTORED DOWN AT 7-1-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
H	815	0	815	0
F	815	0	815	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX /MIN	COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
H	635	408 / 0	108 / 0	0 / 0	0 / 0	120 / 0	0 / 0
F	635	408 / 0	108 / 0	0 / 0	0 / 0	120 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (CS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (CS)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 32	-84.3 -84.3	0.12 (1)	10.00	G-C	0 / 207	0.05 (3)
B-C	-502 / 0	-84.3 -84.3	0.27 (1)	6.25	B-G	0 / 432	0.11 (1)
C-D	-502 / 0	-84.3 -84.3	0.27 (1)	6.25	G-D	0 / 432	0.11 (1)
D-E	0 / 32	-84.3 -84.3	0.12 (1)	10.00			
H-B	-877 / 0	0.0 0.0	0.08 (1)	7.81			
F-D	-877 / 0	0.0 0.0	0.08 (1)	7.81			
H-I	0 / 0	-31.8 -31.8	0.22 (2)	10.00			
I-G	0 / 0	-31.8 -31.8	0.22 (2)	10.00			
G-J	0 / 0	-31.8 -31.8	0.22 (2)	10.00			
J-F	0 / 0	-31.8 -31.8	0.22 (2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	4-1-0	-172	-172		FRONT	VERT	TOTAL
I	1-0-8	-139	-139		BACK	VERT	TOTAL
J	7-1-8	-139	-139		BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 3.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CPrimeHip

SIDE SETBACK = 4-1-0

END SETBACK = 3-0-0

END WALL WIDTH = 5-8

CORNER FRAMING TYPE: CONVENTIONAL

END JACK TYPE: CONVENTIONAL

APPLIED TO FRONT SIDE

- ADDTL. LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014

- CSA 089-09

- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = 1/360 (0.27")

CALCULATED VERT. DEFL. (LL) = 1/999 (0.02")

ALLOWABLE DEFL. (TL) = 1/360 (0.27")

CALCULATED VERT. DEFL. (TL) = 1/999 (0.03")

CSI: TC=0.27 (C-D:1) , BC=0.22 (F-G:2) , WB=0.11 (B-G:1) , SSI=0.14 (G-H:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00

COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	618	354	1667
	822	2284	1658

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.62 (G) (INPUT = 0.90)

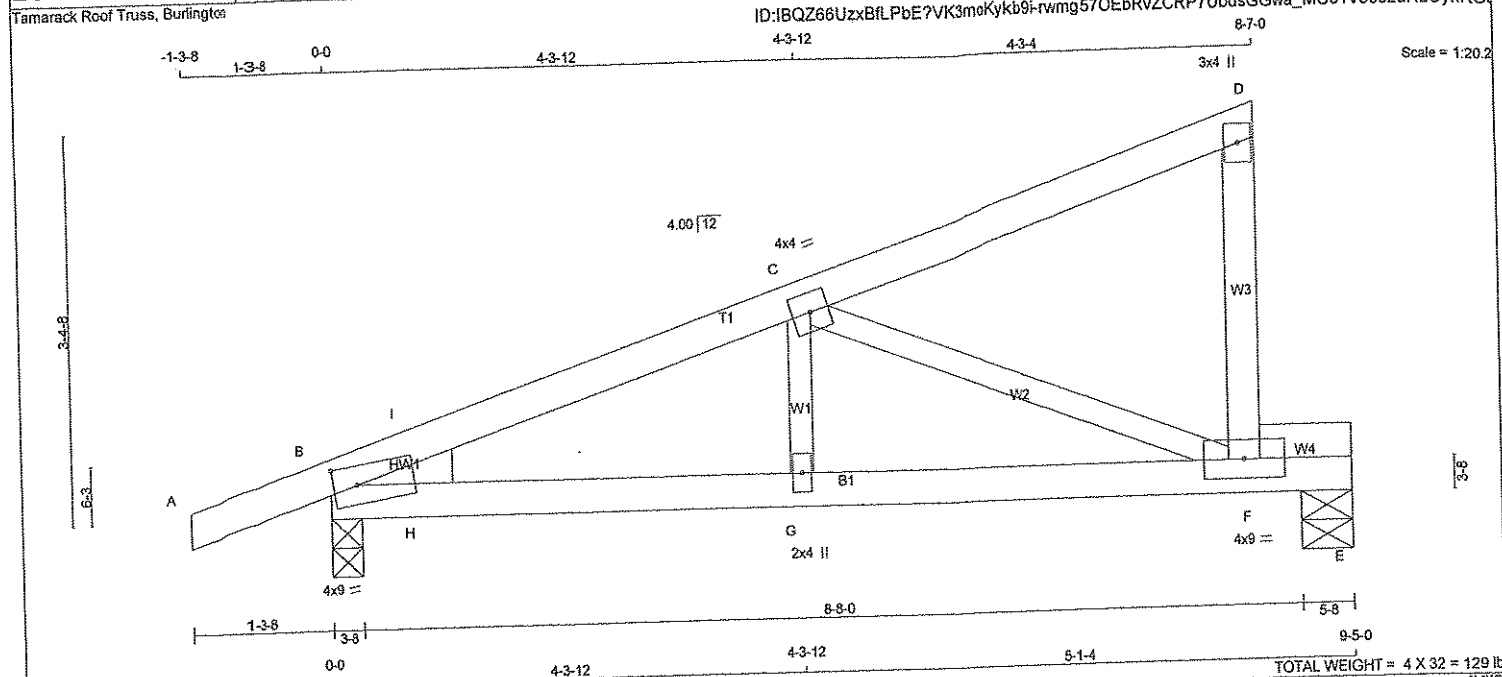
JSI METAL= 0.16 (B) (INPUT = 1.00)



DWG NO. TAM 4327517

STRUCTURAL

COMPONENT ONLY



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY
 DRY: SEASONED LUMBER

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-m	MT20	4.0	9.0	2.00	2.75
C	TMBW-t	MT20	4.0	4.0		
D	TMBW-p	MT20	3.0	4.0		
F	BMVW-t	MT20	4.0	9.0		
G	BMVW-w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG	HEEL WEDGE
JT VERT	638	0	3-8	2x4 L
B VERT	638	0	3-8	
E VERT	461	0	5-8	

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
JT	508	307 / 0	99 / 0	0 / 0	0 / 0
B	388	200 / 0	99 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.18 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 10	-84.3 -84.3	0.10 (1)	10.00	G-C	0 / 332	0.07 (2)
B-I	-1000 / 0	-84.3 -84.3	0.07 (1)	6.18	C-F	-955 / 0	0.31 (1)
I-C	-932 / 0	-84.3 -84.3	0.14 (1)	6.25	H-I	0 / 82	0.00 (1)
C-D	-9 / 0	-84.3 -84.3	0.17 (1)	10.00			
F-D	-151 / 0	0.0	0.03 (1)	7.81			
B-H	0 / 890	-28.0	-28.0	0.16 (1)	10.00		
H-G	0 / 890	-28.0	-28.0	0.32 (1)	10.00		
G-F	0 / 890	-28.0	-28.0	0.65 (1)	10.00		
F-E	0 / 0	-28.0	-28.0	0.21 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 25.6 PSF
 DL = 3.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.31")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
 ALLOWABLE DEFL.(TL) = L/360 (0.31")
 CALCULATED VERT. DEFL.(TL) = L/642 (0.18")

CSI: TC=0.17 (C-D:1), BC=0.65 (F-G:1), WB=0.31 (C-F:1), SI=0.18 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10
 COMPANION LIVE LOAD FACTOR = 0.50

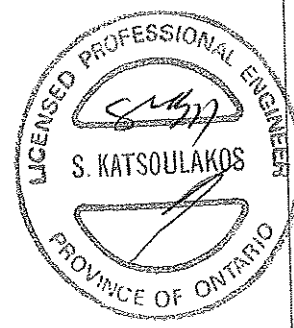
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION
 (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1657 822 2284 1656

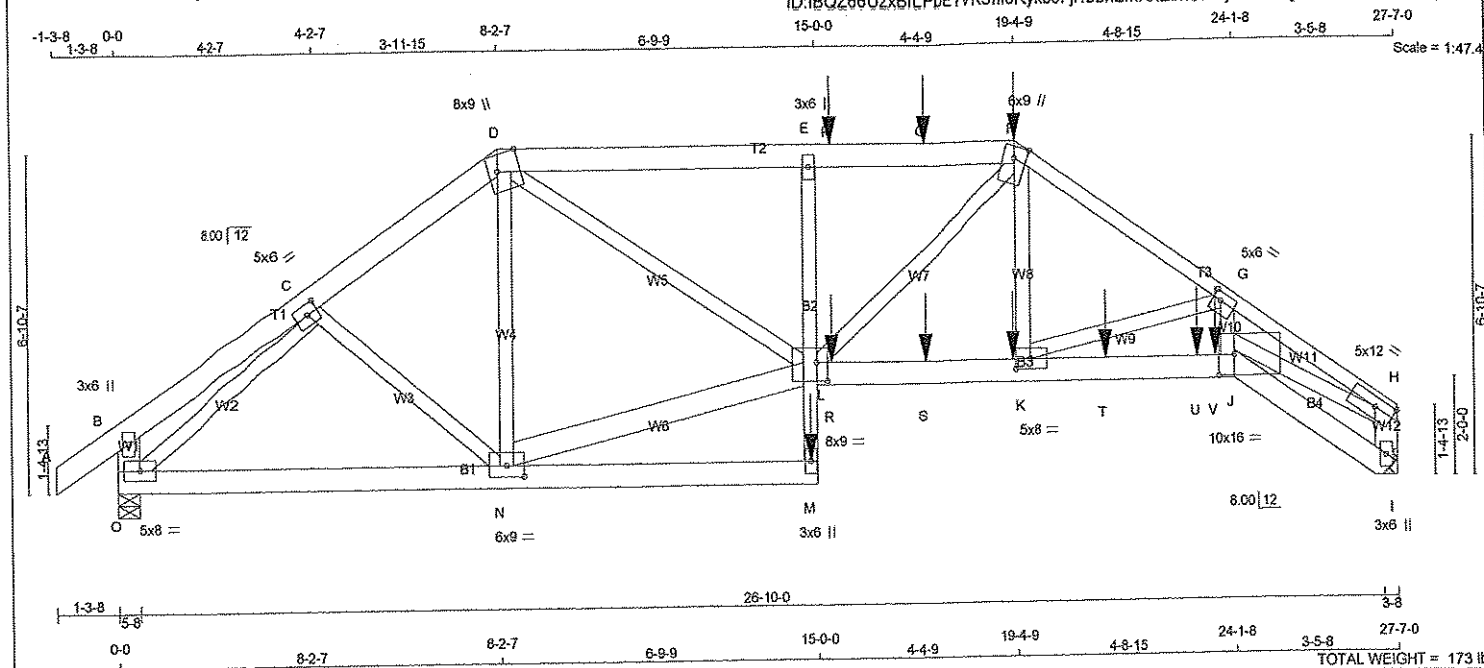
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.64 (C) (INPUT = 0.90)
 JSI METAL= 0.31 (C) (INPUT = 1.00)



DWG NO. TAM 4327617
 STRUCTURAL
 COMPONENT ONLY



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x6	DRY	No.2
D - F	2x6	DRY	No.2
F - H	2x4	DRY	1650F 1.5E
I - H	2x6	DRY	No.2
O - B	2x6	DRY	No.2
O - M	2x6	DRY	No.2
M - E	2x4	DRY	No.2
L - J	2x6	DRY	No.2
J - I	2x6	DRY	No.2
ALL WEBS EXCEPT	2x4	DRY	No.2
N - L	2x6	DRY	No.2
J - H	2x4	DRY	1650F 1.5E

DRY: SEASONED LUMBER

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	3.0	6.0
C	TMVWV-I	MT20	5.0	6.0 2.50 2.75
D	TTVWV+m	MT20	8.0	9.0 Edge 5.75
E	TMV+p	MT20	3.0	6.0
F	TTVWV+m	MT20	6.0	9.0 Edge
G	TMVWV-I	MT20	5.0	6.0 2.00 2.00
H	TMVWV-I	MT20	5.0	12.0 2.25 5.00
I	BMV1+p	MT20	3.0	6.0
J	BMVWV-p	MT20	10.0	16.0 5.00 4.00
K	BMVWV-I	MT20	5.0	8.0 2.50 3.50
L	BMVWVWV-I	MT20	8.0	9.0 4.50 2.75
M	BMV+p	MT20	3.0	6.0
N	BMVWVWV-I	MT20	6.0	9.0 2.75 4.50
O	BMVWV-I	MT20	5.0	8.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 333.6 lbs FACTORED DOWN AT 19-4-9, AND 38.3 lbs FACTORED DOWN AT 15-3-13, AND 38.3 lbs FACTORED DOWN AT 17-3-13 ON TOP CHORD, AND 684.7 lbs FACTORED DOWN AT 14-10-4, 47.3 lbs FACTORED DOWN AT 15-3-13, 34.1 lbs FACTORED DOWN AT 17-3-13, 34.1 lbs FACTORED DOWN AT 19-3-13, 34.1 lbs FACTORED DOWN AT 21-3-13, AND 38.9 lbs FACTORED DOWN AT 23-3-13, AND 288.6 lbs FACTORED DOWN AT 23-3-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	HORZ	BRG	IN-SX	BRG	IN-SX
I	2509	0	0	2509	0	0	0	0	0
O	2178	0	0	2178	0	0	0	0	0

ALLOW FOR 0.4" OF HORIZONTAL MOVEMENT DUE TO TOTAL LOAD

UNFACTORED REACTIONS							
JT	1ST LCASE	MAX. MIN.	COMPONENT REACTIONS	SNOW	LIVE	PERM LIVE	WIND
I	2033	1171 / 0	433 / 0	0 / 0	0 / 0	0 / 0	428 / 0
O	1759	1024 / 0	368 / 0	0 / 0	0 / 0	0 / 0	386 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.62 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED	FORCE	VERT. LOAD	MEMB.	MAX. FACTORED	FORCE	MAX.
FR-TO	(LBS)	(PLF)	LC1 (LC)	FR-TO	(LBS)	(LC)	(LC)
A-B	0 / 33	-84.3	-84.3 0.07 (1)	C-N	0 / 153	0.03 (3)	0.20 (1)
B-C	-3 / 14	-84.3	-84.3 0.09 (1)	D-L	-389 / 47	0.28 (1)	0.44 (1)
C-D	-2521 / 0	-84.3	-84.3 0.14 (1)	E-L	0 / 2146	0.44 (1)	0.17 (1)
D-E	-4175 / 0	-84.3	-84.3 0.39 (1)	F-L	0 / 973	0.17 (1)	0.18 (1)
E-P	-4187 / 0	-84.3	-84.3 0.31 (1)	G-L	0 / 997	0.18 (1)	0.40 (1)
P-Q	-4187 / 0	-84.3	-84.3 0.31 (1)	H-L	0 / 2285	0.40 (1)	0.84 (1)
Q-F	-4187 / 0	-84.3	-84.3 0.31 (1)	I-L	0 / 6535	0.84 (1)	0.89 (1)
F-G	-4228 / 0	-84.3	-84.3 0.48 (1)	J-L	-2742 / 0	0.89 (1)	
G-H	-7525 / 0	-84.3	-84.3 0.55 (1)				
H-I	-2460 / 0	0.0	0.0 0.18 (1)				
O-B	-264 / 0	0.0	0.0 0.02 (1)				
O-N	0 / 2049	-28.0	-28.0 0.43 (2)				
N-M	0 / 24	-28.0	-28.0 0.25 (3)				
M-L	0 / 730	0.0	0.0 0.20 (1)				
L-E	-634 / 0	0.0	0.0 0.15 (1)				
L-R	0 / 3492	-28.0	-28.0 0.52 (1)				
R-S	0 / 3492	-28.0	-28.0 0.52 (1)				
S-K	0 / 3492	-28.0	-28.0 0.52 (1)				
K-T	0 / 6025	-28.0	-28.0 0.89 (1)				
T-U	0 / 6025	-28.0	-28.0 0.89 (1)				
U-V	0 / 6025	-28.0	-28.0 0.89 (1)				
V-J	0 / 6025	-28.0	-28.0 0.89 (1)				
J-I	0 / 0	-28.0	-28.0 0.05 (2)				

FACTORED CONCENTRATED LOADS (LBS)				FACE		DIR.	
JT	LOC.	LC1	MAX-	MAX+	FRONT	VERT	TOTAL
F	19-4-9	-28	-29	-	FRONT	VERT	TOTAL
F	19-4-9	-42	-42	-	FRONT	VERT	TOTAL
F	19-4-9	-266	-266	-	FRONT	VERT	TOTAL
K	19-3-13	-20	-34	-	FRONT	VERT	TOTAL
M	14-10-4	-665	-665	-	FRONT	VERT	TOTAL
P	15-3-13	-38	-38	-	FRONT	VERT	TOTAL
Q	17-3-13	-38	-38	-	FRONT	VERT	TOTAL
R	15-3-13	-27	-47	-	FRONT	VERT	TOTAL
S	17-3-13	-20	-34	-	FRONT	VERT	TOTAL
T	21-3-13	-20	-34	-	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN. GIC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDTL. USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.92")
CALCULATED VERT. DEFL. (LL) = L/999 (0.25")
ALLOWABLE DEFL. (TL) = L/360 (0.92")
CALCULATED VERT. DEFL. (TL) = L/840 (0.39")

CSI: TC=0.55 (G-H-1), BC=0.89 (J-K-1), WB=0.89 (C-O-1), SSI=0.22 (J-K-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	MAX MIN
MT20	618	354	1667 822 2284 1858

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.95 (H) (INPUT = 1.00)

DWG NO. TAM 4328217
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
284266	T20	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID: iBQZ66Uzx8fLPbE?VK3moKykb9-H4bbhZmXtEbhCmFjD4scFDyNB507JrFO6S3EykK3k

FACTORED CONCENTRATED LOADS (LBS)

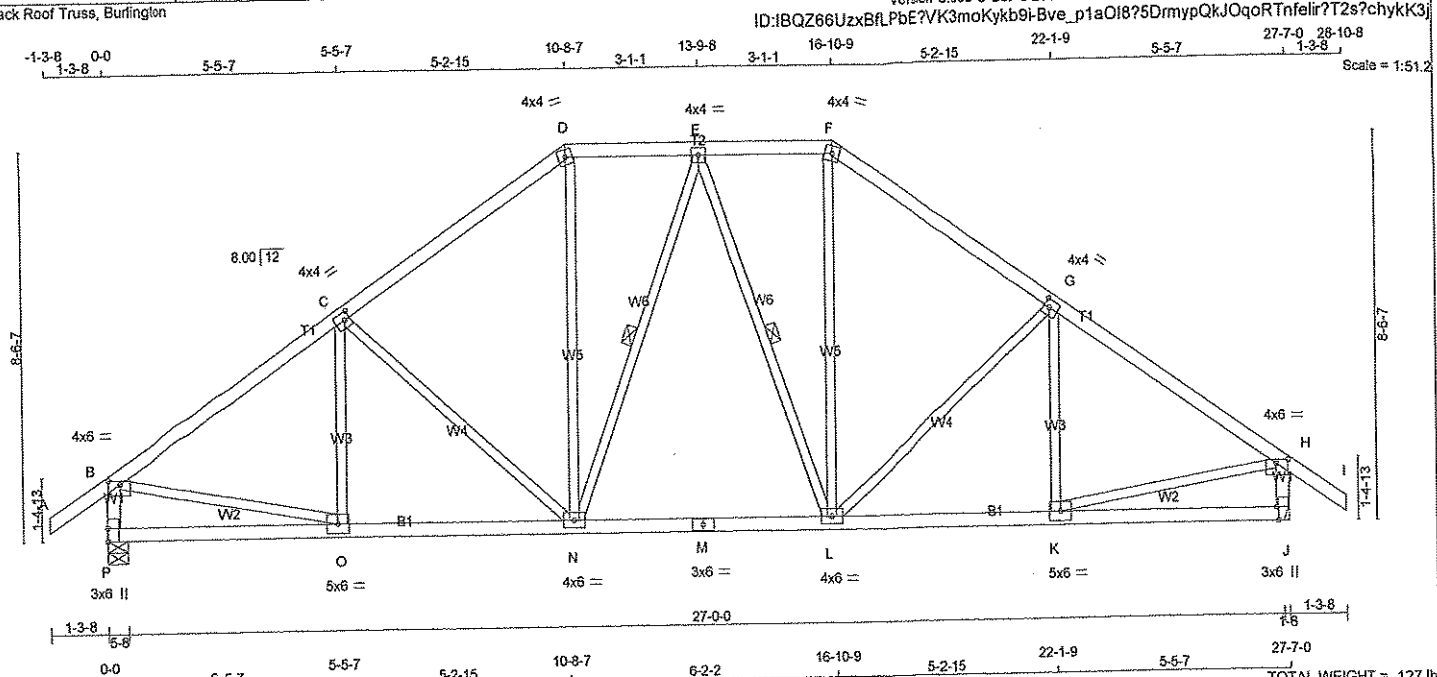
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
U	23-3-13	-22	-39	---	FRONT	VERT	TOTAL
V	23-8-8	-289	-289	---	FRONT	VERT	TOTAL



PM

DWG NO. TAM 4328217

STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS

SIZE	LUMBER	DESCR.
A - D	2x4 DRY	No.2 SPF
D - F	2x4 DRY	No.2 SPF
F - I	2x4 DRY	No.2 SPF
P - B	2x4 DRY	No.2 SPF
J - H	2x4 DRY	No.2 SPF
P - M	2x4 DRY	No.2 SPF
M - J	2x4 DRY	No.2 SPF

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

DRY: SEASONED LUMBER

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVV-p	MT20	4.0	6.0	1.00	3.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	4.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TMVV-p	MT20	4.0	6.0	1.00	3.25
J	BMV1-t	MT20	3.0	6.0	Edge	0.50
K	BMWW-t	MT20	5.0	6.0		
L	BMWWW-t	MT20	4.0	6.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	5.0	6.0		
P	BMV1-t	MT20	3.0	6.0	3.50	

Edge - INDICATES REFERENCE CORNER OF PLATE

TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
VERT	HORZ	DOWN	HORZ
JT 1664	0	1664	0
P 1664	0	1664	0

UPLIFT IN-SX 5-8

HANGER BY OTHERS MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT COMBINED	SNOW					
P 1350	776 / 0	290 / 0	0 / 0	0 / 0	284 / 0	0 / 0
J 1350	776 / 0	290 / 0	0 / 0	0 / 0	284 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.64 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N, E-L

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT.	MAX	MEMB.	FORCE (LBS)	MAX	MAX
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 32	-84.3	-84.3 0.11 (1)	10.00	O-C	-137 / 142	0.05 (1)
B-C	-1787 / 0	-84.3	-84.3 0.37 (1)	4.64	C-N	-368 / 0	0.36 (1)
C-D	-1519 / 0	-84.3	-84.3 0.34 (1)	4.97	N-D	0 / 551	0.12 (1)
D-E	-1244 / 0	-84.3	-84.3 0.10 (1)	5.67	N-E	-159 / 0	0.08 (1)
E-F	-1244 / 0	-84.3	-84.3 0.10 (1)	5.67	E-L	-159 / 0	0.08 (1)
F-G	-1519 / 0	-84.3	-84.3 0.34 (1)	4.97	L-F	0 / 551	0.12 (1)
G-H	-1787 / 0	-84.3	-84.3 0.37 (1)	4.64	L-G	-368 / 0	0.36 (1)
H-I	0 / 32	-84.3	-84.3 0.11 (1)	10.00	K-G	-137 / 142	0.05 (1)
P-B	-1600 / 0	0.0	0.0 0.16 (1)	6.53	B-O	0 / 1541	0.35 (1)
J-H	-1600 / 0	0.0	0.0 0.16 (1)	6.53	K-H	0 / 1541	0.35 (1)
P-O	0 / 0	-28.0	-28.0 0.19 (3)	10.00			
O-N	0 / 1511	-28.0	-28.0 0.36 (2)	10.00			
N-M	0 / 1298	-28.0	-28.0 0.33 (2)	10.00			
M-L	0 / 1298	-28.0	-28.0 0.33 (2)	10.00			
L-K	0 / 1511	-28.0	-28.0 0.36 (2)	10.00			
K-J	0 / 0	-28.0	-28.0 0.19 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 3.0 PSF

BOT CH. LL = 10.6 PSF

DL = 7.0 PSF

TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, CBC 2012, ABC 2014

- CSA 088-09

- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.92")

CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")

ALLOWABLE DEFL.(TL)= L/360 (0.92")

CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.37 (B-C:1), BC=0.36 (N-O:2), WB=0.36 (C-N:1), SSI=0.18 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

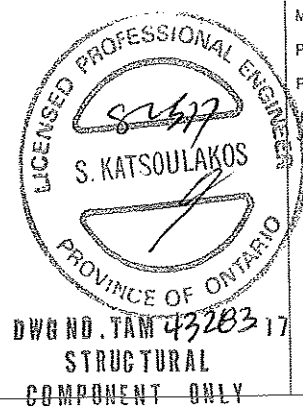
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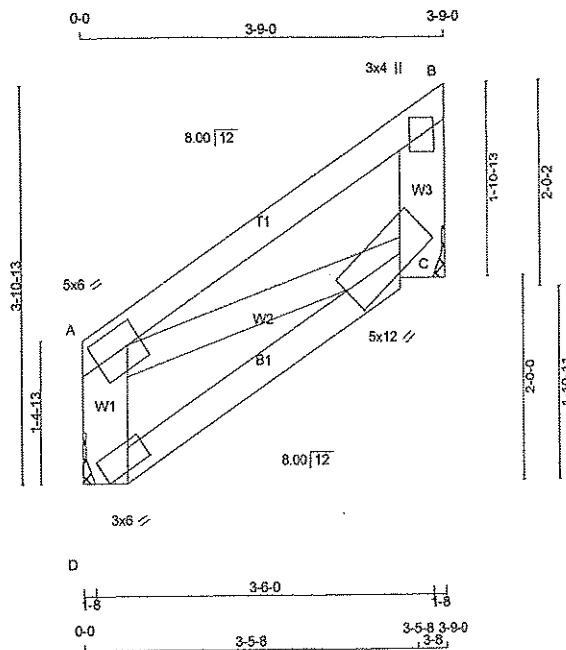
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg

SSI GRIP= 0.86 (B) (INPUT = 0.90)

SSI METAL= 0.44 (M) (INPUT = 1.00)





Scale = 1:22.9

TOTAL WEIGHT = 18 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	SPF
D - A	2x6	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
C - B	2x6	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF
DRY: SEASONED LUMBER				

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS					
FACTORED GROSS REACTION			MAXIMUM FACTORED GROSS REACTION		
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
D	210	0	210	0	0
C	210	0	210	0	0

INPUT REQD
BRG BRG
IN-SX IN-SX
HANGER BY OTHERS
MIN. SEAT SIZE: 1-8
HANGER BY OTHERS
MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS

1ST LCASE		MAX /MIN. COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
D	173	96 / 0	39 / 0	0 / 0	0 / 0	38 / 0
C	173	96 / 0	39 / 0	0 / 0	0 / 0	38 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS			WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)
FR-TO		FROM TO	FR-TO		FROM TO
D-A	-158 / 0	0.0 0.0 0.01 (1)	A-C	0 / 0	0.00 (1)
A-B	0 / 0	-84.3 -84.3 0.20 (1)			
C-B	-158 / 0	0.0 0.0 0.01 (1)			
D-C	0 / 0	-28.0 -28.0 0.12 (2)			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.02")
ALLOWABLE DEFL. (TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.03")

CSI: TC=0.20 (A-B:1), BC=0.12 (C-D:2), WB=0.00 (A-C:1), SSI=0.10 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

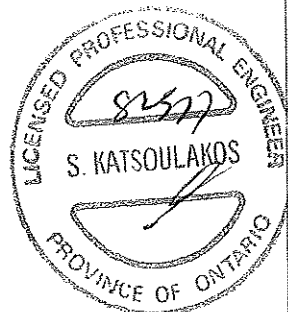
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

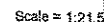
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.07 (D) (INPUT = 0.90)
JSI METAL= 0.05 (D) (INPUT = 1.00)



DWG NO. TAM 4328417
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 18 lb

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.20 (A-B:1), BC=0.12 (C-D:2), WB=0.00 (A-C:1), SSI=0.10 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

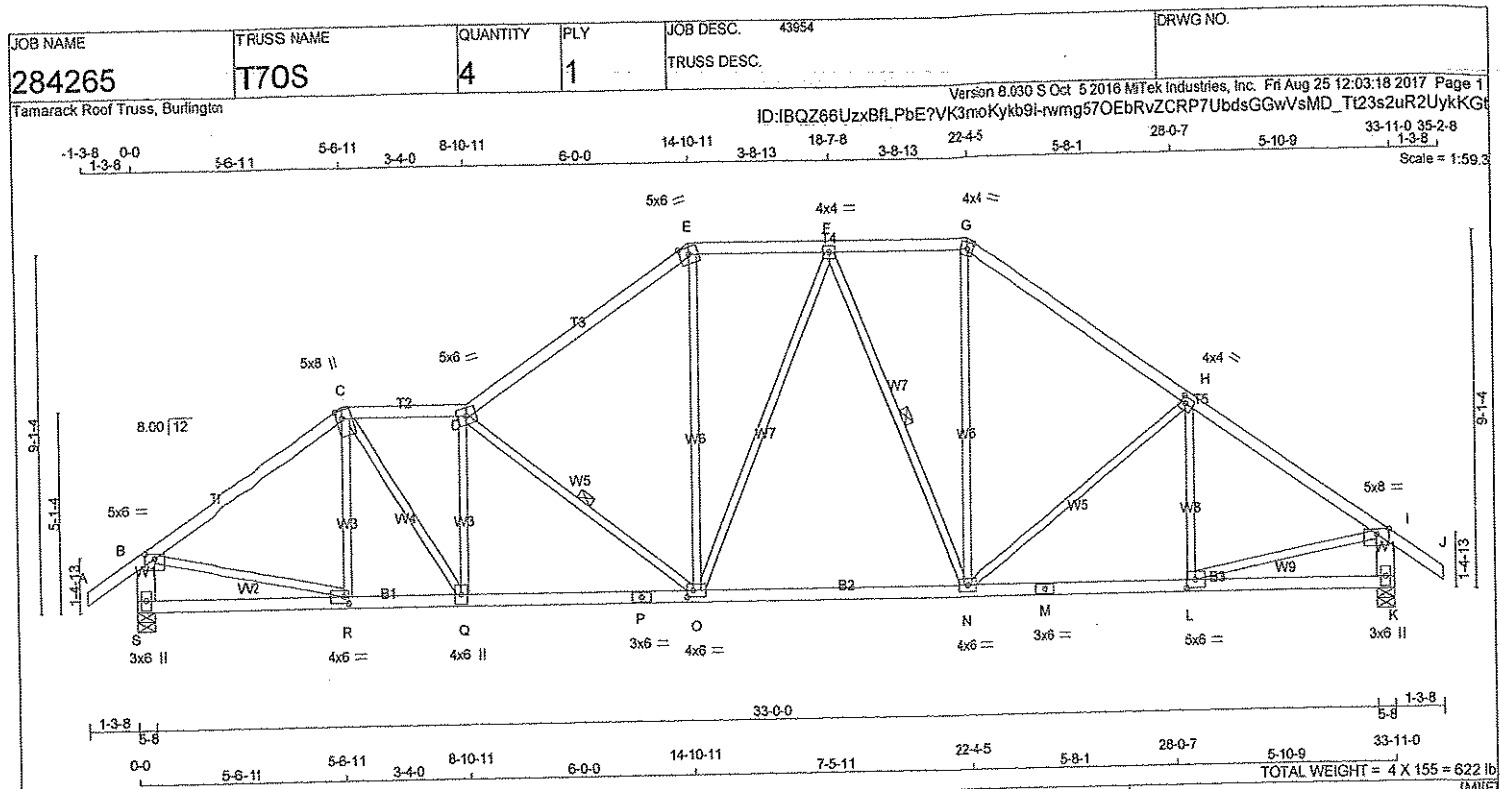
NAIL VALUES					
	PLATE GRIP(DRY)		SHEAR		SECTION
	(PSI)		(PLI)		(PLI)
	MAX	MIN	MAX	MIN	MAX MIN
MT20	618	354	1667	822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. ≈ 5.0 Deg.

JSI GRIP= 0.07 (D) (INPUT = 0.90)
JSI METAL = 0.05 (D) (INPUT = 1.00)





LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - J	2x4	DRY	No.2
S - B	2x6	DRY	No.2
K - I	2x6	DRY	No.2
S - P	2x4	DRY	No.2
P - M	2x4	DRY	No.2
M - K	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER

PLATES (table is in inches)			
JT	TYPE	PLATES	W L E N Y X
B	TMWW-p	MT20	5.0 6.0 1.50 3.00
C	TTWW+m	MT20	5.0 8.0 Edge 1.75
D	TTWW-m	MT20	5.0 6.0
E	TTWW-m	MT20	5.0 6.0 Edge
F	TMWW-t	MT20	4.0 4.0
G	TTWW-m	MT20	4.0 4.0 2.00 1.75
H	TMWW-t	MT20	4.0 4.0 2.00 1.50
I	TMWW-p	MT20	5.0 8.0 Edge
L	BMWW-t	MT20	5.0 6.0 2.50 2.75
M	BS-t	MT20	3.0 6.0
N	BMWW-t	MT20	4.0 6.0
O	BMWW-t	MT20	4.0 6.0 2.00 2.00
P	BS-t	MT20	3.0 6.0
Q	BMWW-t	MT20	4.0 6.0
R	BMWW-t	MT20	4.0 6.0 2.00 1.50
S	BMV1+p	MT20	3.0 6.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQ'D
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG
S	2019	0	2019	0	5-8
K	2019	0	2019	0	5-8

UNFACTORED REACTIONS		1ST LCASE	MAX/MIN	COMPONENT REACTIONS	WIND	DEAD	SOIL
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1641	938 / 0	356 / 0	0 / 0	0 / 0	347 / 0	0 / 0
K	1641	938 / 0	356 / 0	0 / 0	0 / 0	347 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.10 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-O, F-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO	LENGTH FR-TO	R-C			
A-B	0 / 32	-84.3 -84.3	0.11 (1)	10.00	-205 / 74	0.08 (1)	
B-C	-2243 / 0	-84.3 -84.3	0.44 (1)	4.16	C-Q	0 / 1299	0.29 (1)
C-D	-2585 / 0	-84.3 -84.3	0.19 (1)	4.15	Q-D	-978 / 0	0.38 (1)
D-E	-2178 / 0	-84.3 -84.3	0.50 (1)	4.14	D-O	-1009 / 0	0.41 (1)
E-F	-1822 / 0	-84.3 -84.3	0.17 (1)	4.82	O-E	0 / 824	0.19 (1)
F-G	-1668 / 0	-84.3 -84.3	0.16 (1)	5.00	O-F	0 / 84	0.02 (3)
G-H	-2024 / 0	-84.3 -84.3	0.48 (1)	4.30	F-N	-403 / 0	0.26 (1)
H-I	-2290 / 0	-84.3 -84.3	0.46 (1)	4.10	N-G	0 / 776	0.17 (1)
I-J	0 / 32	-84.3 -84.3	0.11 (1)	10.00	N-H	-348 / 0	0.43 (1)
S-B	-1953 / 0	0.0 0.0	0.13 (1)	7.20	L-H	-212 / 115	0.08 (1)
K-I	-1956 / 0	0.0 0.0	0.13 (1)	7.20	B-R	0 / 1897	0.43 (1)
					L-I	0 / 1972	0.44 (1)
S-R	0 / 0	-28.0 -28.0	0.19 (2)	10.00			
R-Q	0 / 1859	-28.0 -28.0	0.40 (1)	10.00			
Q-P	0 / 2606	-28.0 -28.0	0.59 (2)	10.00			
P-O	0 / 2606	-28.0 -28.0	0.59 (2)	10.00			
O-N	0 / 1820	-28.0 -28.0	0.50 (2)	10.00			
N-M	0 / 1934	-28.0 -28.0	0.48 (2)	10.00			
M-L	0 / 1934	-28.0 -28.0	0.48 (2)	10.00			
L-K	0 / 0	-28.0 -28.0	0.20 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.13")
CALCULATED VERT. DEFL.(LL) = L/999 (0.15")
ALLOWABLE DEFL.(TL) = L/360 (1.13")
CALCULATED VERT. DEFL.(TL) = L/999 (0.26")

CSI: TC=0.59 (D-E:1), BC=0.59 (O-Q:2), WB=0.44 (I-L:1), SSI=0.20 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

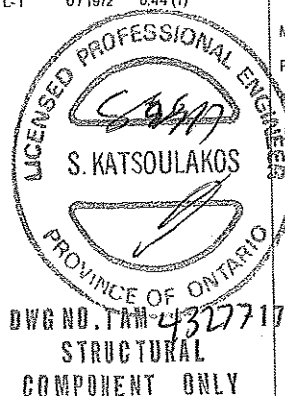
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1587 822 2284 1655

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.89 (R) (INPUT = 0.90)
SI METAL= 0.65 (P) (INPUT = 1.00)

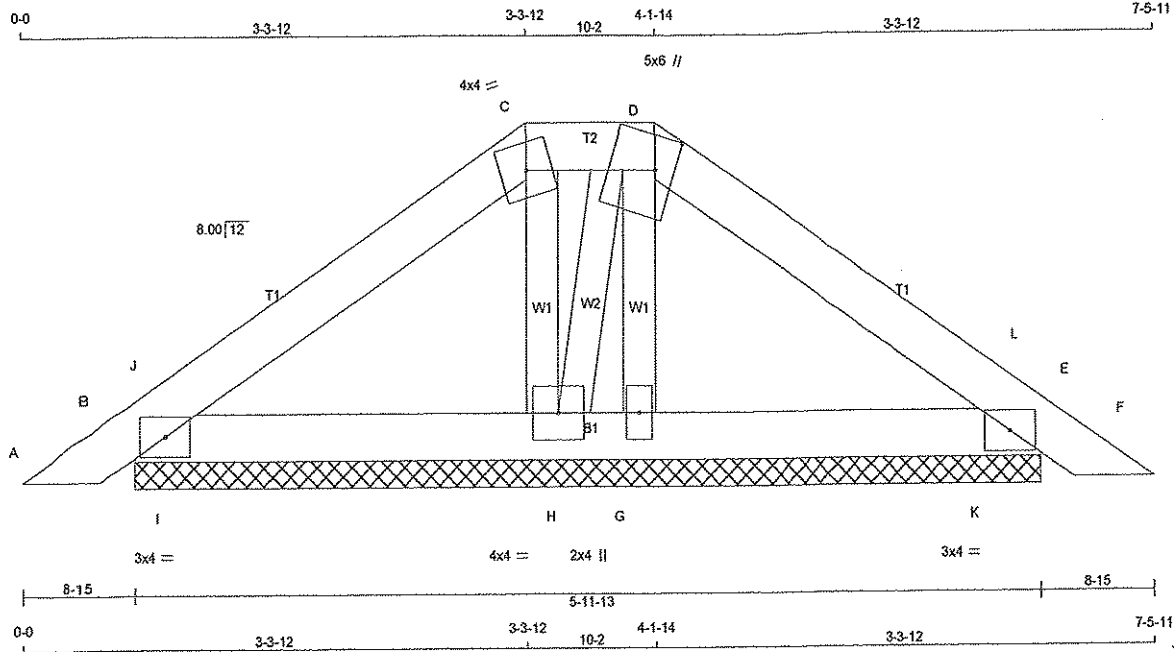


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
284265	P1	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1:14.5

TOTAL WEIGHT = 21 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	SPF
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER				

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMB1-I	MT20	3.0	4.0
C	TTW-m	MT20	4.0	4.0
D	TTWVW+m	MT20	5.0	6.0 2.50 1.50
E	TMB1-I	MT20	3.0	4.0
G	BMW1-w	MT20	2.0	4.0
H	BMWV1-I	MT20	4.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
B	225	0	225	0	5-11-13	5-11-13
E	222	0	222	0	5-11-13	5-11-13
H	217	0	217	0	5-11-13	5-11-13
G	119	0	119	0	5-11-13	5-11-13

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX/MIN COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM.LIVE	WIND			
B	174	114 / 0	28 / 0	0 / 0	0 / 0	32 / 0	0 / 0
E	171	113 / 0	27 / 0	0 / 0	0 / 0	31 / 0	0 / 0
H	179	99 / 0	41 / 0	0 / 0	0 / 0	39 / 0	0 / 0
G	103	49 / 0	29 / 0	0 / 0	0 / 0	25 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)		FACTORED VERT. LOAD LC1 (PLF)		MAX. CS1 (LC)	WEBS MAX. FACTORED FORCE (LBS)		MAX. CS1 (LC)
	FR-TO	FROM TO	FR-TO	FROM TO		FR-TO	FROM TO	
A-B	0 / 14	-84.3	-84.3	0.03 (1)	10.00	H-C	-97 / 0	0.01 (1)
B-J	-58 / 0	-84.3	-84.3	0.02 (3)	6.25	H-D	-44 / 0	0.01 (1)
J-C	-59 / 0	-84.3	-84.3	0.07 (1)	6.25	G-D	-53 / 0	0.01 (1)
C-D	-32 / 0	-84.3	-84.3	0.01 (1)	6.25	I-J	-125 / 50	0.00 (1)
D-L	-60 / 0	-84.3	-84.3	0.07 (1)	6.25	K-L	-135 / 41	0.00 (1)
L-E	-52 / 0	-84.3	-84.3	0.02 (3)	6.25			
E-F	0 / 14	-84.3	-84.3	0.03 (1)	10.00			
B-I	0 / 48	-28.0	-28.0	0.06 (1)	10.00			
I-H	0 / 48	-28.0	-28.0	0.06 (1)	10.00			
H-G	0 / 45	-28.0	-28.0	0.06 (2)	10.00			
G-K	0 / 48	-28.0	-28.0	0.07 (1)	10.00			
K-E	0 / 48	-28.0	-28.0	0.07 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.8	PSF
	DL = 3.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CS1: TC=0.07 (C-J:1), BC=0.07 (G-K:1), WB=0.01 (C-H:1), SSI=0.11 (E-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(ORY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1657
	822	2284	1655

PLATE PLACEMENT TOL. = 0.250 inches

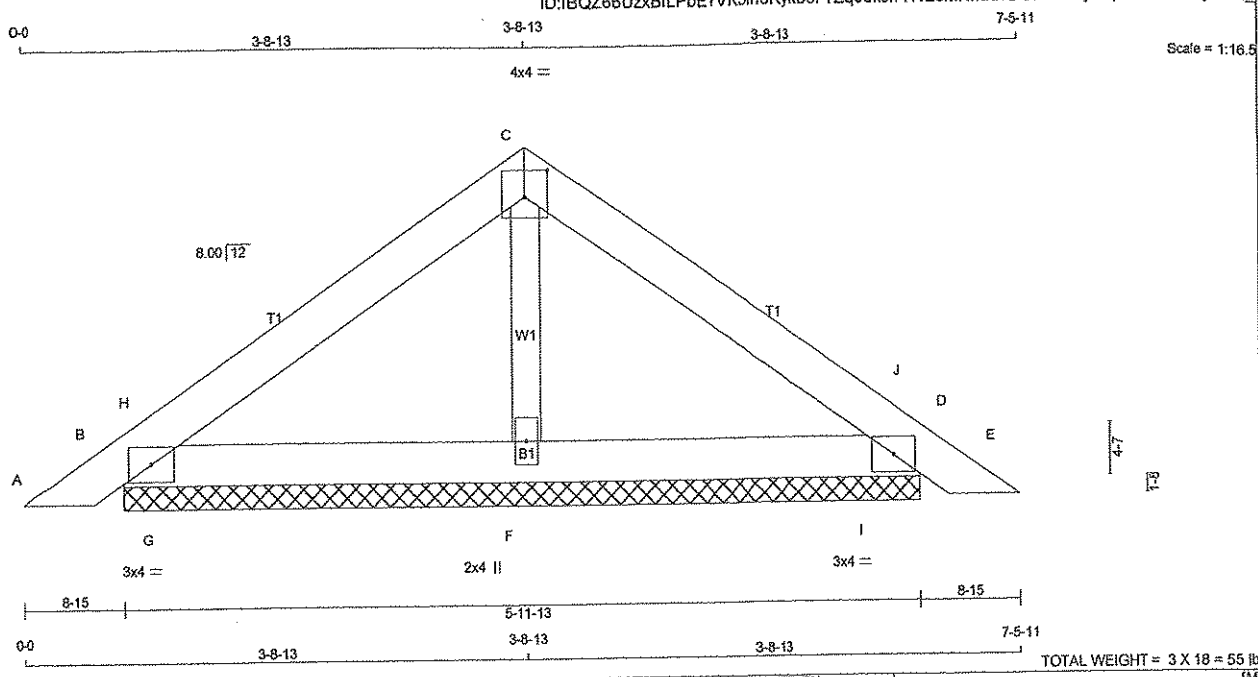
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.16 (E) (INPUT = 0.90)
JSI METAL= 0.04 (E) (INPUT = 1.00)



DRWG NO. TAN 4322017
STRUCTURAL
COMPONENT ONLY

JOB NAME 284265	TRUSS NAME P2	QUANTITY 3	PLY 1	JOB DESC. 43954	DRWG NO.
Tamarack Roof Truss, Burlington					



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	2.25	2.00
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMB1-I	MT20	3.0	4.0	2.25	2.00
F	BMW1+w	MT20	2.0	4.0	2.25	2.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT		REQD	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	BRG	IN-SX	BRG
B	251	0	251	0	0	5-11-13	5-11-13	5-11-13	5-11-13
D	251	0	251	0	0	5-11-13	5-11-13	5-11-13	5-11-13
F	281	0	281	0	0	5-11-13	5-11-13	5-11-13	5-11-13

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	192	130/0	28/0	0/0	0/0	34/0	0/0	0/0
D	192	130/0	28/0	0/0	0/0	34/0	0/0	0/0
F	243	115/0	69/0	0/0	0/0	59/0	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	MEMB.	FORCE (LBS)	LC1 MAX	CSH (LC)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0/14	-84.3	-84.3 0.03 (1)	10.00	F-C	-116/0	0.02 (1)
B-H	-54/0	-84.3	-84.3 0.03 (1)	6.25	G-H	-197/37	0.00 (1)
H-C	-89/0	-84.3	-84.3 0.09 (1)	6.25	I-J	-197/37	0.00 (1)
C-J	-89/0	-84.3	-84.3 0.09 (1)	6.25			
J-D	-54/0	-84.3	-84.3 0.03 (1)	6.25			
D-E	0/14	-84.3	-84.3 0.03 (1)	10.00			
B-G	0/70	-28.0	-28.0 0.09 (1)	10.00			
G-F	0/70	-28.0	-28.0 0.09 (1)	10.00			
F-I	0/70	-28.0	-28.0 0.09 (1)	10.00			
I-D	0/70	-28.0	-28.0 0.09 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 3.0 PSF
BOT CH. LL	= 10.5 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSH: TC=0.09 (C-J:1), BC=0.09 (F-G:1), WB=0.02 (C-F:1), SSI=0.15 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

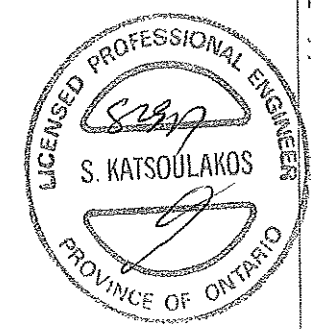
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)



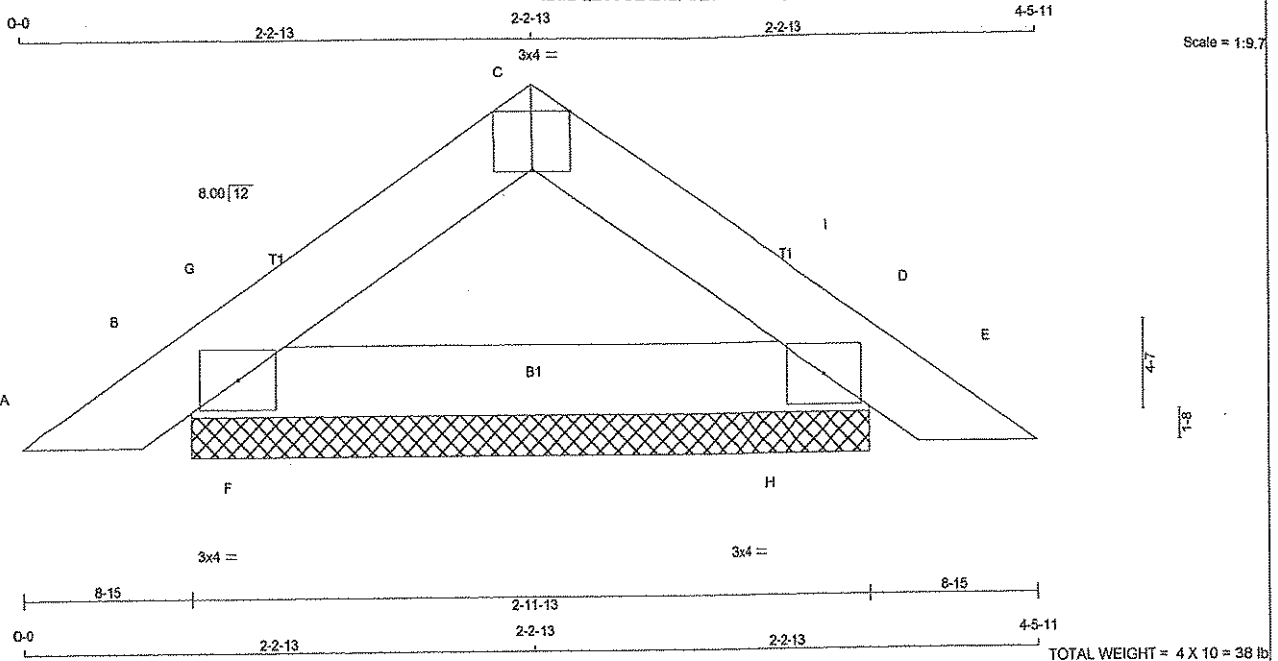
DWG NO. TAM 4327917
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
284266	P3	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 4 X 10 = 38 lb

LUMBER	DESCR.
N. L. G. A. RULES	
CHORDS	SIZE
A - C	2x4 DRY No.2 SPF
C - E	2x4 DRY No.2 SPF
B - D	2x4 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE	PLATES			
B	TMB1-I	MT20	3.0	4.0
C	TT-p	MT20	3.0	4.0
D	TMB1-I	MT20	3.0	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQRD
JT	VERT	HORZ	DOWN	HORZ
B	223	0	223	0
D	223	0	223	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE	
B	175	110/0	31/0	0/0	34/0
D	175	110/0	31/0	0/0	34/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	FACTORED HORZ. LOAD (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	FACTORED HORZ. LOAD (LBS)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0/14	-84.3	-84.3	0.03 (1)	10.00	F-G	0/101
B-G	-169/0	-84.3	-84.3	0.04 (2)	6.25	H-I	0/101
C-C	-110/0	-84.3	-84.3	0.04 (1)	6.25		
C-I	-110/0	-84.3	-84.3	0.04 (1)	6.25		
I-D	-169/0	-84.3	-84.3	0.04 (2)	6.25		
D-E	0/14	-84.3	-84.3	0.03 (1)	10.00		
B-F	0/99	-28.0	-28.0	0.02 (2)	10.00		
F-H	0/99	-28.0	-28.0	0.05 (2)	10.00		
H-D	0/99	-28.0	-28.0	0.02 (2)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.04 (B-G/2), BC=0.05 (F-H/2), WB=0.00 (F-G/1), SSI=0.05 (D-I/2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

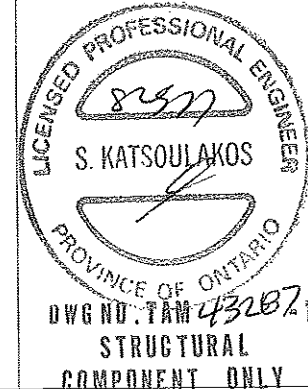
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	618	354	1657
	822	2284	1656

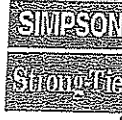
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (D) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)



LUS - Double Shear Joist Hangers



All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

MATERIAL: 18 gauge

FINISH: G90 galvanized

DESIGN:

- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

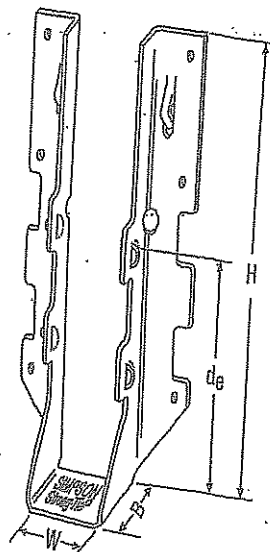
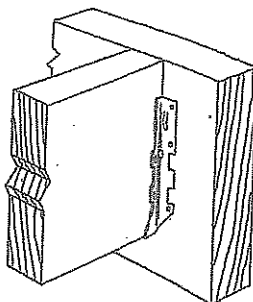
INSTALLATION:

- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

- These hangers cannot be modified.

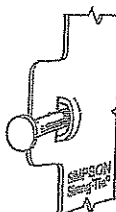
Typical LUS Installation



LUS28

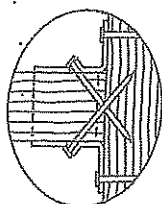
Model No.	Ga	Dimensions (In)				Fasteners		Factored Resistance (lbs)			
								D.Fir-L		S-P-F	
		W	H	B	d _g ¹	Face	Joist	Uplift	Normal	Uplift	Normal
								(K _p =1.15)	(K _p =1.00)	(K _p =1.15)	(K _p =1.00)
LUS24	18	1 1/8	3 1/4	1 1/4	1 1/8	4-10d	2-10d	710	1630	645	1155
LUS24-2	18	3 1/4	3 1/4	2	1 1/8	4-16d	2-16d	835	2020	590	1435
LUS26	18	1 1/8	4 1/4	1 3/4	3/8	4-10d	4-10d	1420	2170	1290	1630
LUS26-2	18	3 1/4	4 1/4	2	4	4-16d	4-16d	1720	2595	1545	1920
LUS26-3	18	4 1/4	4 1/4	2	3/4	4-16d	4-16d	1720	2595	1545	2340
LUS28	18	1 1/8	5 1/4	1 3/4	3/8	6-10d	4-10d	1420	2520	1290	1790
LUS28-2	18	3 1/4	7	2	4	6-16d	4-16d	1720	3325	1545	2575
LUS28-3	18	4 1/4	6 1/4	2	3/4	6-16d	4-16d	1720	3325	1545	2375
LUS210	18	1 1/8	7 1/8	1 3/4	3/8	8-10d	4-10d	1420	2785	1290	2210
LUS210-2	18	3 1/4	9	2	6	8-16d	6-16d	2580	4500	2320	3195
LUS210-3	18	4 1/4	8 1/4	2	5 1/4	8-16d	6-16d	2580	3345	2320	2375

1. d_g is the distance from the seat of the hanger to the highest joist nail.

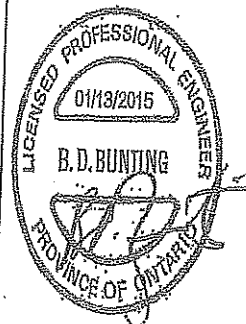


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,589



Double Shear Nailing Top View.



This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2015. This information is updated periodically and should not be relied upon after December 31, 2016. Contact Simpson Strong-Tie for current information and limited warranty or see www.strongtie.com.

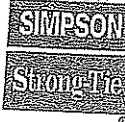
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J-SPEC LUS 15 4/15, Rev. 12/16

800-990-5099

www.strongtie.com

HUS/LJS - Double Shear Joist Hangers



All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: See table

FINISH: G90 galvanized

DESIGN:

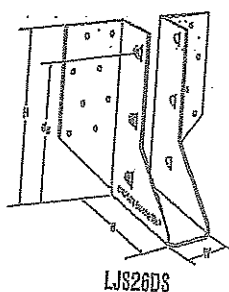
- Factored resistances are in accordance with CSA 086-14
- Uplift resistances have been increased 15%. No further increase is permitted
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

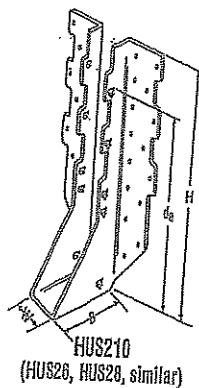
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

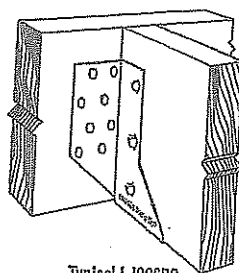
- See current catalogue for options



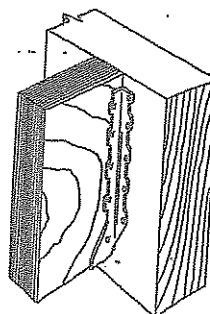
LJS26DS



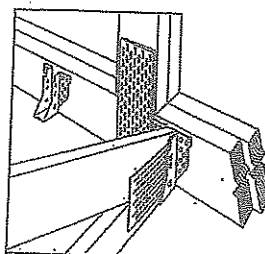
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



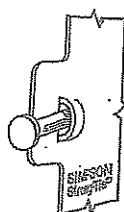
Typical HUS
Installation



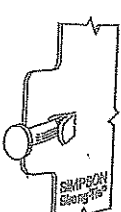
Typical HUS Installation
(Truss Designer to provide fastener
quantity for connecting multiple
members together)

Model No.	Ga	Dimensions (in)		Fasteners		Factored Resistance (lbs)			
						D-Fir-L		S-P-F	
		W	H	B	d _g	Face	Joist	Uplift (K _u =1.15)	Normal (K _u =1.00)
LJS26DS	18	1 1/16	5	3 1/2	4%	16-16d	6-16d	2055	4265
HUS26	18	1%	5%	3	5 1/16	14-16d	6-16d	2705	4940
HUS28	18	1%	7 1/2	3	6 1/2	22-16d	8-16d	3805	6365
HUS210	18	1%	9 1/2	3	7 1/2	30-16d	10-16d	4505	5795
HUS1.81/10	18	1 1/16	9	3	8	30-16d	10-16d	4505	6450

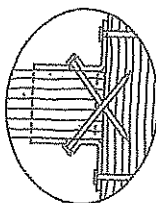
1. d_g is the distance from the seat of the hanger to the highest joist nail.



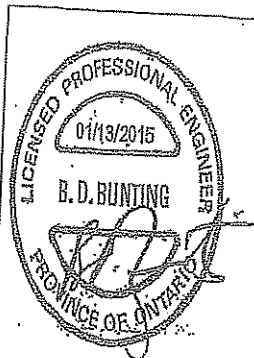
Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



UNIVERSITY OF
STATES
DESIGN

This technical bulletin is a guide only. It is not intended to be used as a substitute for engineering design. The information is provided for general information only. The user is responsible for the proper use of the information. The user is responsible for the proper use of the information. The user is responsible for the proper use of the information.

800-999-6009
www.simpsonstrongtie.com

HGUS - Double Shear Joist Hangers

SIMPSON

Strong-Tie

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: 12 gauge

FINISH: G90 galvanized

DESIGN:

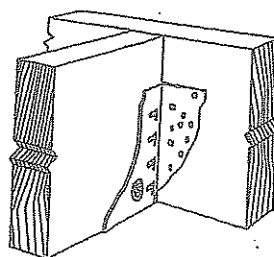
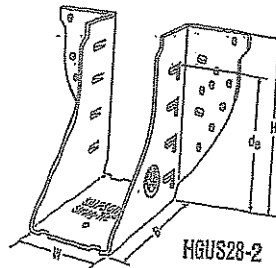
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

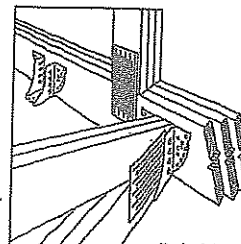
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

- See current catalogue for options.



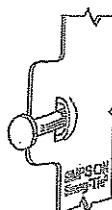
Typical HGUS Installation



Typical HGUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

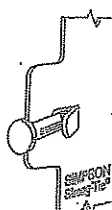
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistances (lbs)			
		W	H	B	d _p ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _p =1.15)	Normal (K _p =1.00)	Uplift (K _p =1.15)	Normal (K _p =1.00)
HGUS26	12	1 1/2	5 1/2	5	4 1/2	20-16d	8-16d	2685	6625	2685	5700
HGUS26-2	12	3 1/2	6 1/2	4	4 1/2	20-16d	8-16d	4385	8950	3100	6355
HGUS26-3	12	4 1/2	5 1/2	4	4 1/2	20-16d	8-16d	4385	8950	3100	6355
HGUS26-4	12	6 1/2	5 1/2	4	4 1/2	20-16d	8-16d	4385	8950	3100	6355
HGUS28	12	1 1/2	7 1/2	5	6 1/2	36-16d	12-16d	3310	7675	3100	6900
HGUS28-2	12	3 1/2	7 1/2	4	6 1/2	36-16d	12-16d	6070	12980	4310	9215
HGUS28-3	12	4 1/2	7 1/2	4	6 1/2	36-16d	12-16d	6070	12980	4310	9215
HGUS28-4	12	6 1/2	7 1/2	4	6 1/2	36-16d	12-16d	6070	12980	4310	9215
HGUS210-2	12	3 1/2	8 1/2	4	8 1/2	46-16d	16-16d	6840	14645	4855	10400
HGUS210-3	12	4 1/2	8 1/2	4	8 1/2	46-16d	16-16d	6840	14645	4855	10400
HGUS210-4	12	6 1/2	8 1/2	4	8 1/2	46-16d	16-16d	6840	14645	4855	10400
HGUS212-4	12	6 1/2	10 1/2	4	10 1/2	58-16d	20-16d	7640	14995	5425	10645
HGUS214-4	12	6 1/2	12 1/2	4	11 1/2	68-16d	22-16d	10130	16400	7195	11045

1. d_p is the distance from the seat of the hanger to the highest joist nail.

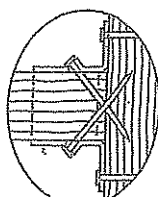


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

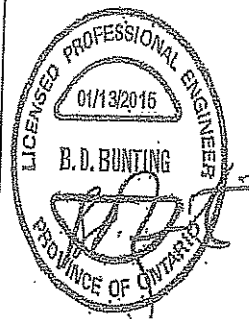
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMITED WARRANTY
DESIGN

This document is a technical bulletin and does not constitute a contract. It is intended to provide general information only and should not be relied upon for specific design or construction requirements. For complete information, refer to the Simpson Strong-Tie catalog and technical specifications.

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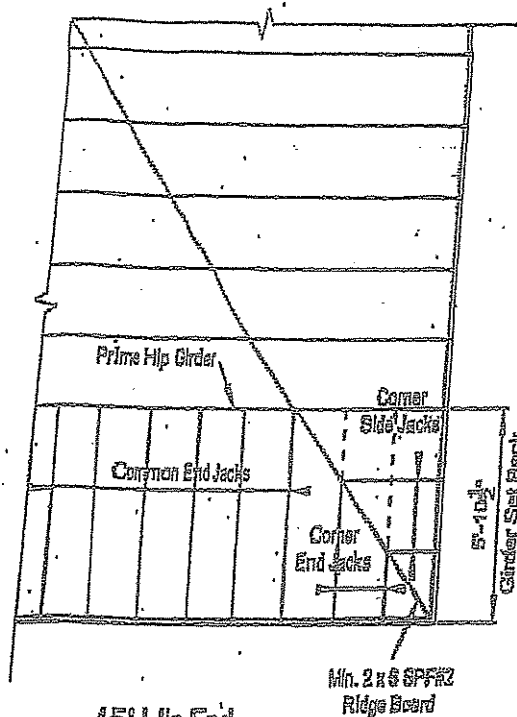
15-SPEC-HGUS-16 1/15 Rev. 12/16

800-999-5099
www.strongtie.com

MICRO CITY

ENGINEERING SERVICES INC.

TEL: (519) 287-2242



45° Hip End

LUMBER SPECIFICATION

TOP CHORD : 2x4 SPF#2

BOTTOM CHORD : 2x4 SPF#2

WEBS : 2x3 SPF#2

UNLESS OTHERWISE SHOWN

DESIGN LOAD:

TOP CHORD LIVE LOAD : 39.4 P.S.F.

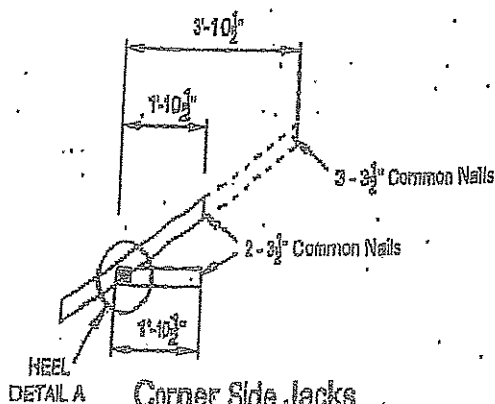
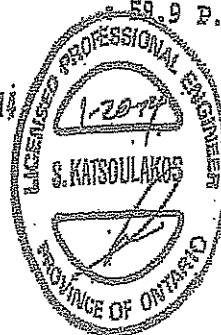
TOP CHORD DEAD LOAD : 3.0 P.S.F.

BOTTOM CHORD LIVE LOAD : 10.5 P.S.F.

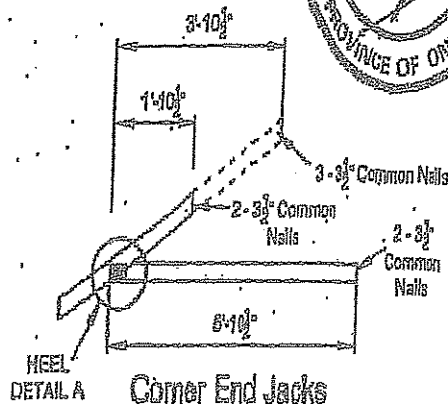
BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD : 59.9 P.S.F.

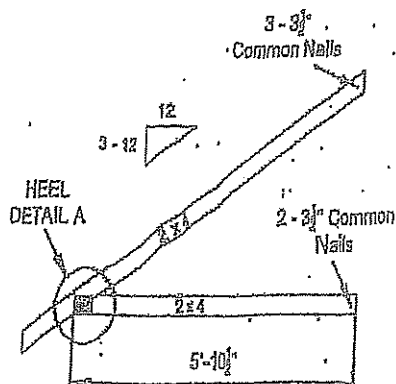
DWG NO. T118495.11
STRUCTURAL
COMPONENT ONLY



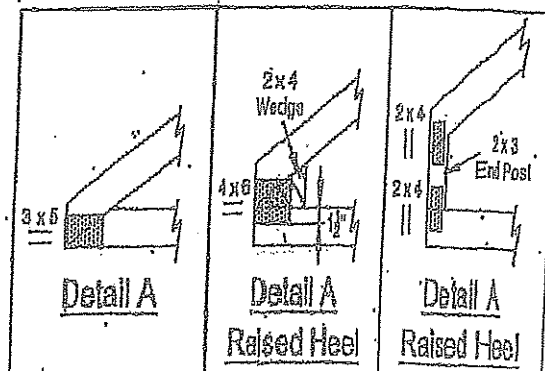
Corner Side Jacks



Corner End Jacks



Common End Jacks



Detail A

Detail A

Detail A

Raised Heel

Raised Heel

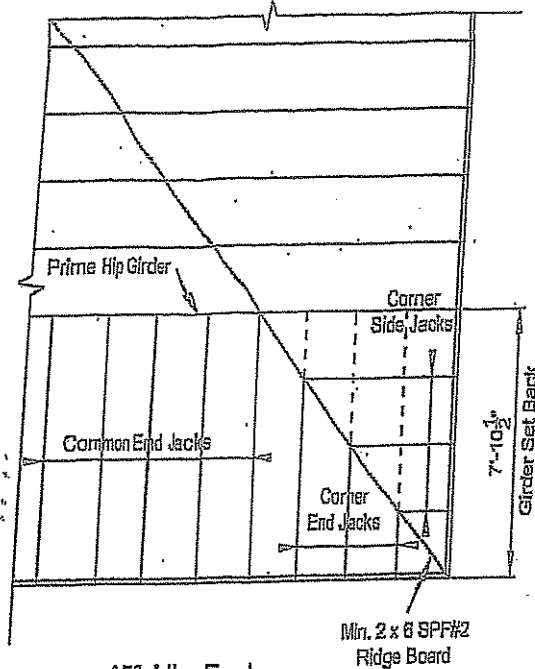
LUMBER SPECIFICATION

TOP CHORD : 2x4 SPF#2
 BOTTOM CHORD : 2x4 SPF#2
 WEBS : 2x3 SPF#2
 UNLESS OTHERWISE SHOWN

DESIGN LOAD:

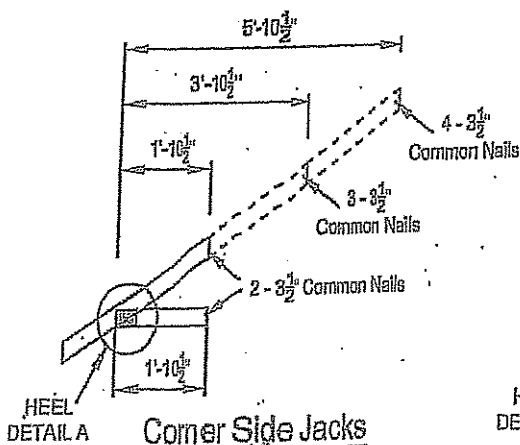
TOP CHORD LIVE LOAD : 34.8 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD : 44.8 P.S.F.

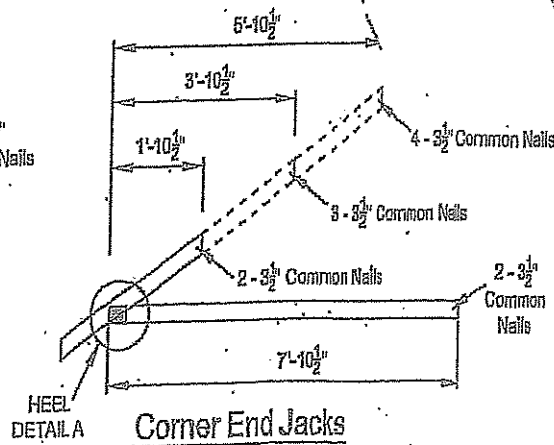


45° Hip End

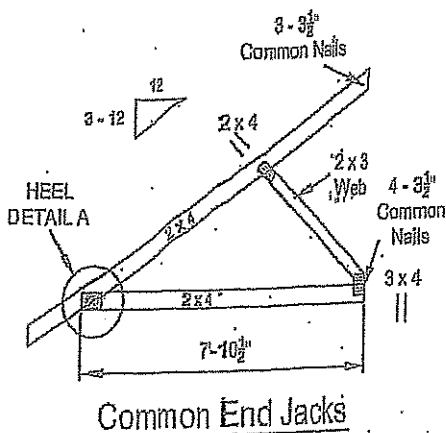
DWG NO. TAM 350314
 STRUCTURAL
 COMPONENT ONLY



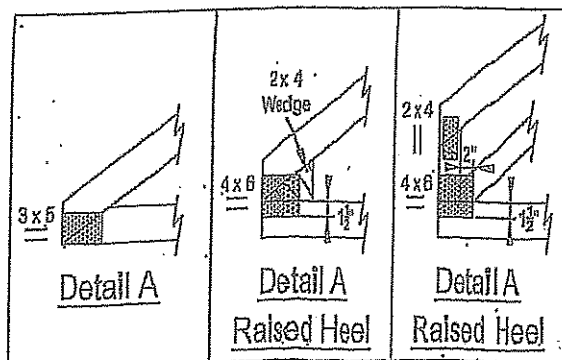
Corner Side Jacks



Corner End Jacks



Common End Jacks



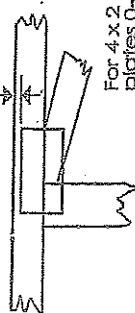
NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
 (TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

Symbols

PLATE LOCATION AND ORIENTATION

Center plate on joint unless x, y offsets are indicated. Dimensions are in 1/16-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.

0-1/16"



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

* Plate location details available in Mitek software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



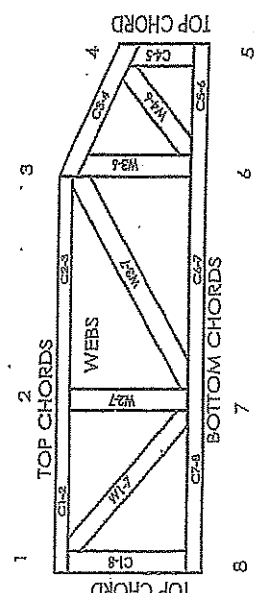
Indicates location where bearings (supports) occur. Icons vary, but reaction section indicates joint number where bearings occur.

Industry Standards:

- TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
- DSB-89: Design Standard for Bracing
- BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in 1/16-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

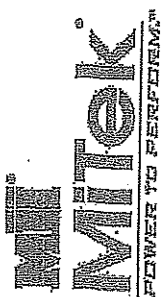
CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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Mitek Engineering Reference Sheet: MII-7473C rev. 10-'08

General Safety Notes

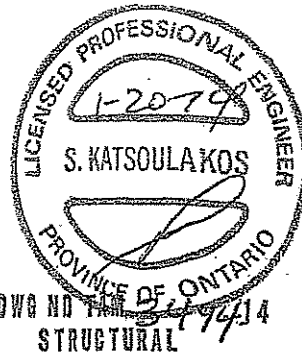
Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loadings shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and ensure fully, knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
9. Unless expressly noted, this design is not applicable for responsibility of truss fabrication. General practice is to camber for dead load deflection.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

Micro City Engineering Services Inc.
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Responsibilities:

Micro City Engineering Services is responsible for the design of trusses as individual components.

It is the responsibilities of others to ascertain that the design loads utilized on this (these) drawing(s) meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdiction over such decisions.

All dimensions are to be verified by the owner, contractor, architect, or other authority having input over such decisions prior to truss component manufacture. At no time shall Micro City Engineering Services Inc. or its employees be responsible for dimension errors.

Micro City Engineering Services Inc. bears no responsibility for the erection of any truss components. Persons erecting truss components are cautioned to seek professional advice regarding temporary and permanent bracing systems and to be totally familiar with all aspects of truss erection prior to proceeding on any truss component erection job. Any bracing shown on Micro City Engineering Services Inc. or Tamarack Roof Trusses Inc. sealed or unsealed truss component drawings is specified for the single truss component in question and is identified as an integral part of the design for that particular truss component but is not meant to represent the only required bracing for that particular truss component when installed as a component in a series of truss components in a roof truss system.

~~It is the truss manufacturer's responsibility to ensure that trusses are manufactured in accordance with Micro City Engineering Services Inc. specifications outlined below:~~

SPECIFICATIONS:

Truss components sealed by Micro City Engineering Services Inc. must conform to the relevant sections of the current Building Code of Ontario and Canada (Part 4 or Part 9) or the current Farm Building Code of Canada in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the Truss Plate Institute of Canada (TPIC). All unit lumber and nailing stresses identified on truss component design drawings and/or used in the design of individual truss components shall conform to the current CSA Wood Design standard identified in the current Building Code and TPIC Design Standards.

The lumber used to manufacture any truss component is to conform to the specified size and grade identified on the truss drawing.

The lumber used in the manufacture of any truss component is not to exceed 19% during its service use unless specifically noted on the truss drawing.

The lumber used in the manufacture of any truss component is not to be treated with any chemicals during its service life unless specifically noted on the truss drawing.

Connector plates shall be applied to both faces of the truss component at each joint and shall be positioned exactly as specified.

The top chord of any truss component is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the sealed truss component drawing but not exceeding 24" o/c (Part 9 design) and not exceeding 48" o/c (Part 4 or Agricultural design).

When a truss component is to be installed with no rigid ceiling attached directly to the bottom chord, then the bottom chord is to be laterally braced at intervals not exceeding 3m (or 10'-0").

All sealed or unsealed truss component drawings provided by Micro City Engineering Services Inc. Or Tamarack Roof Trusses Inc. should be read in conjunction with the following:

Warning-Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473C rev 10-'08 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer - not the truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component Safety Information available from the Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA, 22314.