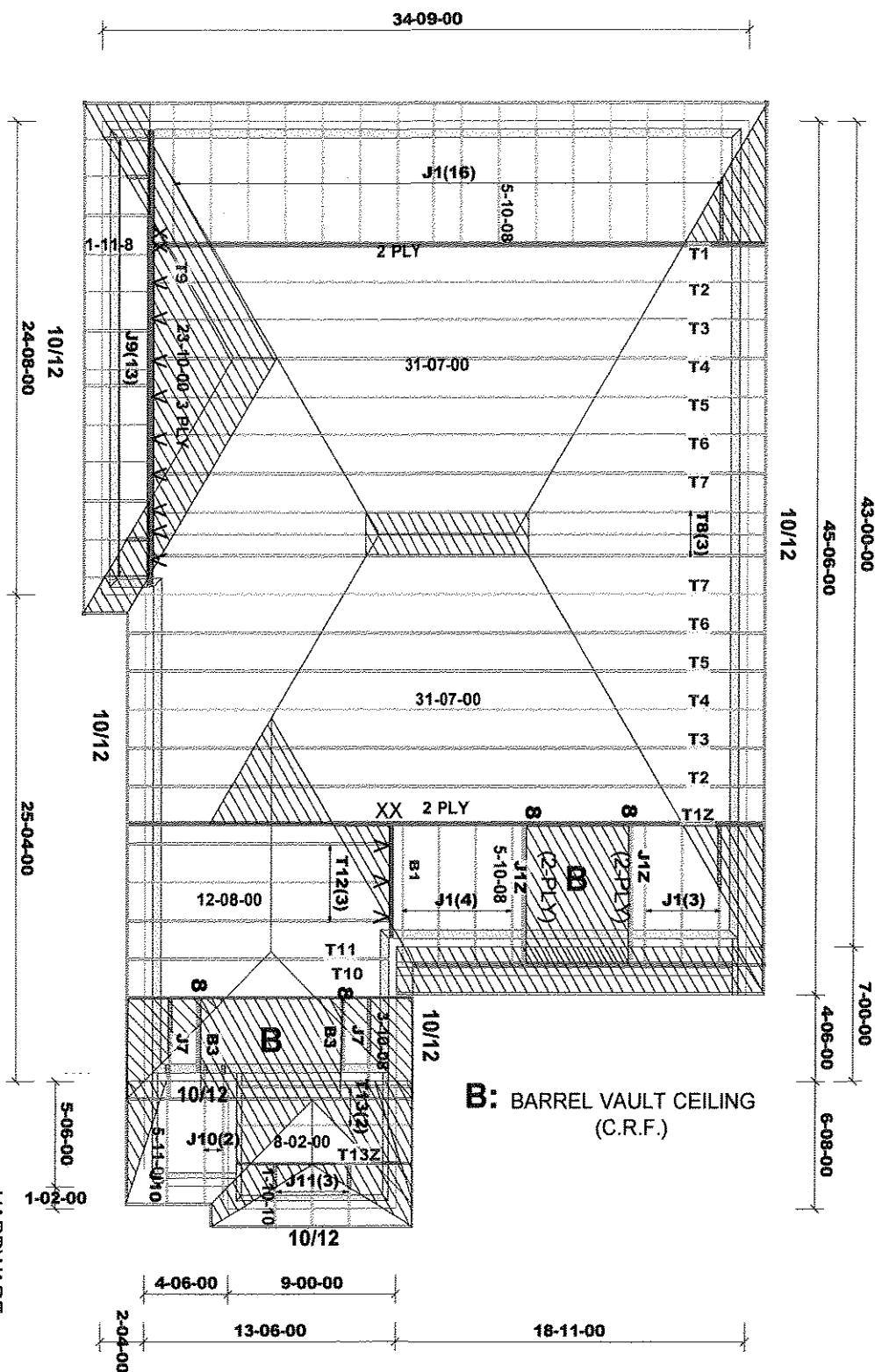




ASPHALT SHINGLES
12" FINISHED O.H.
5" CLADDING ALLOWANCE
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD
HEEL: R.T.M.C.

ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. 2012 ROOF RAFTERS THAT CROSS OVER OR MEET TRUSSES TO BE 2X4 SPF #2 24" O.C. WITH A VERT. POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6" TO HAVE LATERAL BRACING SO THAT THE DIST. BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

DESIGN CONFORMS WITH OBC 2012 OCCUPANCY: RESIDENTIAL | PART: 9 GROUND SNOW LOAD Ss=31.3psf S_r=8.4psf DESIGN LOADS
TCDL (3psf)
BCLL (10.5psf)
BCDL (7psf)

BEAMS:
B1-2 - 2X10 SPF #2
B3: 2 - 2X8 SPF #2

NOTES:
CONVENTIONAL FRAMING

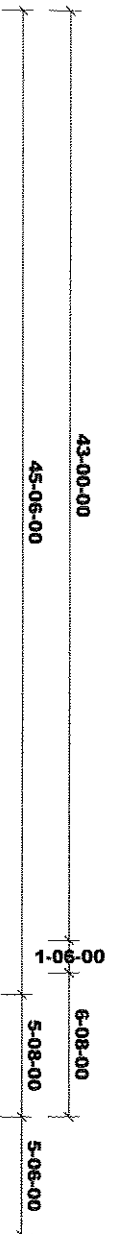
ALL ROOF SLOPES ARE 6/12 UNLESS OTHERWISE NOTED

HARDWARE:

HGUS26-2 -(XX)
LUS26DS -(V)
LUS24-2 -(8)

1109077

		Job Track: 43954		Builder / Location: GREENPARK HOMES / WATERDOWN		Model / Elevation: ROSEWOOD 2 / 1	
Layout ID: 282689		Project: RUSSEL GARDENS PHASE 2		THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.			
Plan Log: 92461		Date: 8/24/2017		Designer: colin/s.v.e		PURPOSE:	





Job Track: 43954
Layout ID: 282695
Plan Log: 92461

Builder / Location:
GREENPARK HOMES / WATERDOWN

Model / Elevation:
ROSEWOOD 2 / 3

Project: **RUSSEL GARDENS PHASE 2**
Date: 8/24/2017
Designer: cjohns.V.E
THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES, INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES, INC. AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES, INC. IF UTILIZED FOR ANY OTHER PURPOSE.
MARK: ver 7.5.0

MD 977

T - 170375

ASPHALT SHINGLES
12" FINISHED O.H.
5" CLADDING ALLOWANCE
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. 2012 ROOF RAFTERS THAT CROSS OVER OR MEET TRUSSES TO BE 2x4 SPF #2 24" O.C. WITH A VERT. POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DIST. BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

DESIGN CONFORMS WITH OBC 2012 OCCUPANCY: RESIDENTIAL | PART: 9 GROUND SNOW LOAD Ss=31.3psf S=8.4psf DESIGN LOADS
TCDL (3psf)
BCLL (10.5psf)
BCDL (7psf)

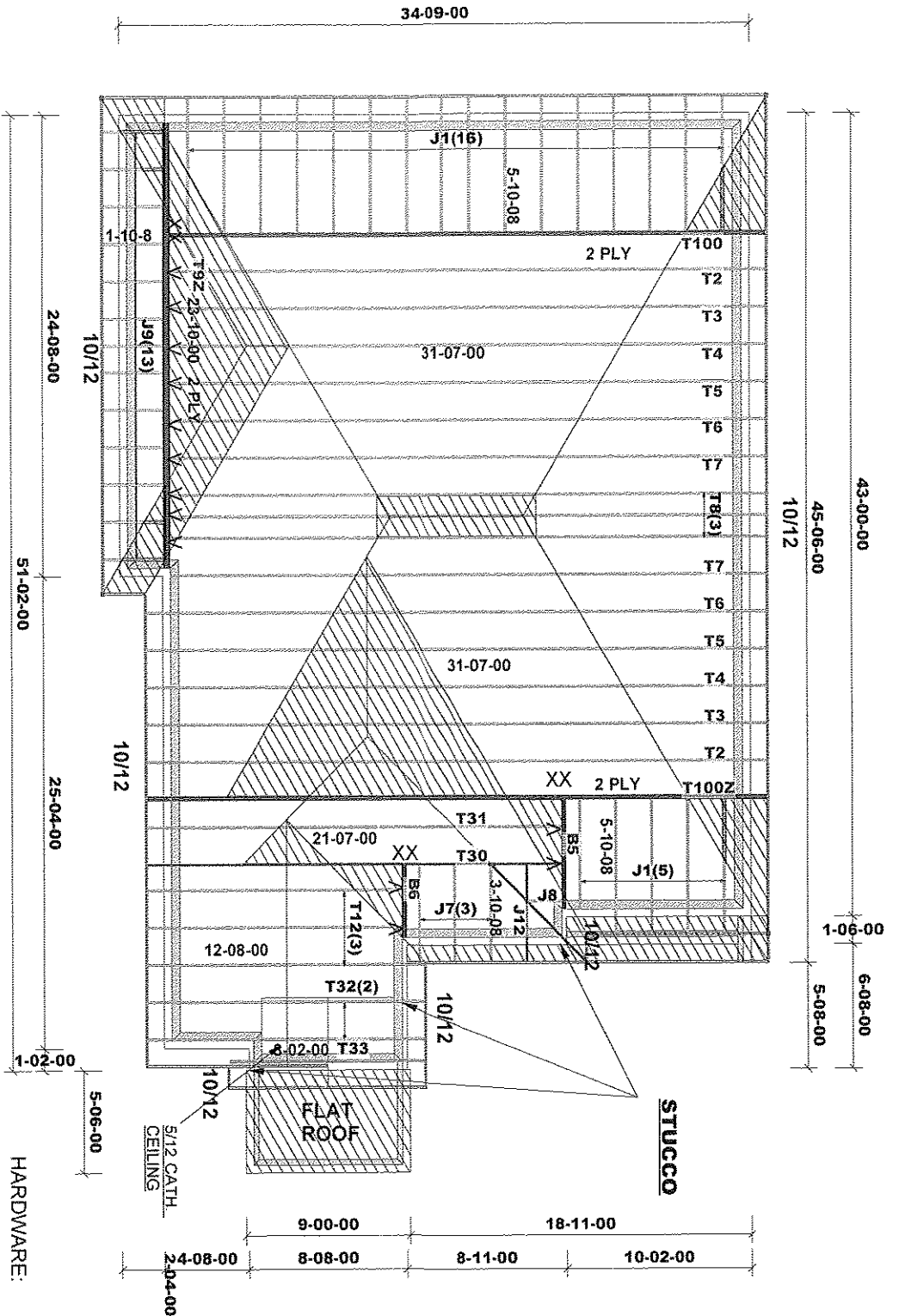
BEAMS:
ALL BEAMS (B): 2 - 2x10 SPF #2

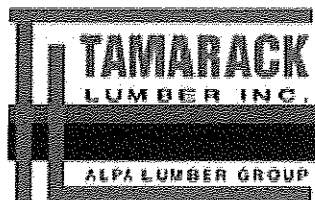
HARDWARE:

HGUS26-2 -(XX)
LJS26DS -(V)

CONVENTIONAL FRAMING

ALL ROOF SLOPES ARE 6/12 UNLESS OTHERWISE NOTED





Delivery Shiplist

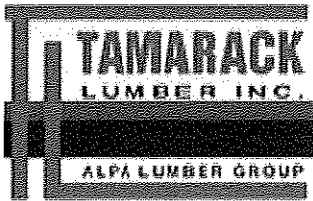
DATE	08/24/17
SALES REP	Mario

JOB TRACK:43954 LAYOUT ID: 282689 LOCATION: WATERDOWN
 BUILDER: GREENPARK - RUSSELL GARDENS PH 2 SUB-BUILDER:
 MODEL: ROSEWOOD 2 ELEVATION: 1

ROOF TRUSSES

ROOF TRUSS SPACING:24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	T1 HIP GIRDER	10.00	31-07-00	04-01-04	2 X 6	2 X 6	01-03-08	01-07-11	365.44		
	2 Ply		0.00							216.00		
	1	T1Z HIP GIRDER	10.00	31-07-00	04-01-04	2 X 6	2 X 6	01-03-08	01-07-11	365.44		
	2 Ply		0.00							216.00		
	2	T2 HIP	10.00	31-07-00	05-01-04	2 X 4	2 X 4	01-03-08	01-07-11	260.78		
			0.00							168.34		
	2	T3 HIP	10.00	31-07-00	06-01-04	2 X 4	2 X 4	01-03-08	01-07-11	261.04		
			0.00							163.00		
	2	T4 HIP	10.00	31-07-00	07-01-04	2 X 4	2 X 4	01-03-08	01-07-11	284.48		
			0.00							177.66		
	2	T5 HIP	10.00	31-07-00	08-01-04	2 X 4	2 X 4	01-03-08	01-07-11	299.10		
			0.00							191.34		
	2	T6 HIP	10.00	31-07-00	09-01-04	2 X 4	2 X 4	01-03-08	01-07-11	303.02		
			0.00							188.34		
	2	T7 HIP	10.00	31-07-00	10-01-04	2 X 4	2 X 4	01-03-08	01-07-11	325.86		
			0.00							204.00		
	3	T8 HIP	10.00	31-07-00	11-01-04	2 X 4	2 X 4	01-03-08	01-07-11	505.26		
			0.00							312.99		
	1	T9 HIP GIRDER	6.00	23-10-00	03-03-04	2 X 4	2 X 6	00-00-00	01-02-00	382.08		
	3 Ply		0.00							236.01		
	1	T10 HIP GIRDER	10.00	12-08-00	04-10-07	2 X 4	2 X 4	01-03-08	01-07-11	59.74		
			0.00							38.00		
	1	T11 HIP	10.00	12-08-00	06-06-07	2 X 4	2 X 4	01-03-08	01-07-11	63.96		
			0.00							40.33		
	3	T12 COMMON	10.00	12-08-00	06-11-00	2 X 4	2 X 4	01-03-08	01-07-11	182.28		
			0.00							118.50		
	2	T13 COMMON	6.00	08-02-00	02-04-11	2 X 4	2 X 4	01-03-08	00-04-03	49.58		
			0.00							32.66		
	1	T13Z COMMON	6.00	08-02-00	02-04-11	2 X 4	2 X 4	01-03-08	00-04-03	24.79		
			0.00							16.33		
	23	J1 JACK-OPEN	6.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08	01-02-00	386.17		
			0.00							245.41		
	2	J1Z JACK-OPEN	6.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08	01-02-00	67.16		
	2 Ply		0.00							42.68		
	2	J7 JACK-OPEN	10.00	03-10-08	04-10-07	2 X 4	2 X 4	01-03-08	01-07-11	27.50		
			0.00							17.34		



Delivery Shiplist

DATE	08/24/17
SALES REP	Mario

JOB TRACK:43954 LAYOUT ID: 282689 LOCATION: WATERDOWN
 BUILDER: GREENPARK - RUSSELL GARDENS PH2 SUB-BUILDER:
 MODEL: ROSEWOOD 2 ELEVATION: 1

ROOF TRUSSES

ROOF TRUSS SPACING:24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	13	J9 JACK-OPEN	10.00 0.00	01-11-08	03-03-04	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 03-03-04	113.36 78.00		
	3	J10 JACK-OPEN	6.00 0.00	05-11-00	03-03-11	2 X 4	2 X 4	01-03-08 00-00-00	00-04-03 03-03-11	48.30 30.00		
	3	J11 JACK-OPEN	10.00 0.00	01-10-10	02-04-11	2 X 4	2 X 4	01-03-08 00-00-00	00-09-13 02-04-11	22.77 15.99		

TOTAL # TRUSS= 78.00

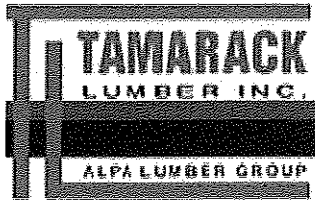
TOTAL BFT OF ALL TRUSSES=

2748.92 BFT. TOTAL WEIGHT OF ALL TRUSSES= 4398.11 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
4	Hangers	LUS24-2	
12	Hangers	LJS26DS	

TOTAL # ITEMS= 18.00



Delivery Shiplist

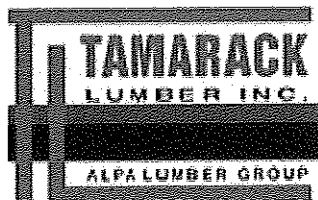
DATE	08/24/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 282690 LOCATION: WATERDOWN
 BUILDER: GREENPARK - RUSSELL GARDENS PH 2 SUB-BUILDER:
 MODEL: ROSEWOOD 2 ELEVATION: 2

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY PLY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER TOP BOT	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	1	T15	6.00	33-11-00	04-01-04	2 X 4 2 X 6	01-03-08	01-02-00	359.64		
	2 Ply	HIP GIRDER	0.00				01-03-08	01-02-00	224.00		
	1	T16	6.00	33-11-00	05-01-04	2 X 4 2 X 4	01-03-08	01-02-00	133.28		
		HIP	0.00				01-03-08	01-02-00	84.67		
	1	T17	6.00	33-11-00	06-01-04	2 X 4 2 X 4	01-03-08	01-02-00	135.85		
		HIP	0.00				01-03-08	01-02-00	85.67		
	1	T18	6.00	33-11-00	07-01-04	2 X 4 2 X 4	01-03-08	01-02-00	141.99		
		HIP	0.00				01-03-08	01-02-00	89.17		
	1	T19	0.00	33-11-00	05-04-04	2 X 6 2 X 6	00-00-00	05-04-04	656.37		
	3 Ply	FLAT GIRDER	0.00				00-00-00	05-04-04	393.99		
	12	T21	6.00	29-05-00	10-08-08	2 X 4 2 X 4	01-03-08	01-02-00	1639.68		
		COMMON	0.00				00-00-00	05-06-08	1032.00		
	5	T20	6.00	27-11-00	10-04-00	2 X 4 2 X 4	01-03-08	01-02-00	626.50		
		COMMON	0.00				00-00-00	05-06-08	390.85		
	1	G20	6.00	27-11-00	10-04-00	2 X 4 2 X 4	01-03-08	01-02-00	141.87		
		GABLE	0.00				00-00-00	05-06-08	88.67		
	1	T10Z	10.00	12-08-00	04-10-07	2 X 4 2 X 4	01-03-08	01-07-11	59.74		
		HIP GIRDER	0.00				01-03-08	01-07-11	38.00		
	1	T23	10.00	12-08-00	06-00-03	2 X 6 2 X 6	00-00-00	01-07-11	174.14		
	2 Ply	HIP GIRDER	0.00				00-00-00	01-07-11	109.34		
	3	T25S	10.00	08-02-00	05-00-08	2 X 4 2 X 4	01-03-08	01-07-11	124.17		
		ROOF	10.00				01-03-08	01-07-11	81.99		
	1	T22	6.00	09-07-00	05-09-00	2 X 4 2 X 4	01-03-08	01-02-00	45.15		
		COMMON	0.00				00-00-00	00-03-08	28.33		
	1	G22	6.00	09-07-00	05-09-00	2 X 4 2 X 4	01-03-08	01-02-00	43.62		
		GABLE	0.00				00-00-00	00-03-08	28.33		
	12	J1	6.00	05-10-08	04-01-04	2 X 4 2 X 4	01-03-08	01-02-00	201.48		
		JACK-OPEN	0.00				00-00-00	04-01-04	128.04		
	2	J2	6.00	05-10-08	02-00-12	2 X 4 2 X 4	01-03-08	01-02-00	32.32		
		JACK-OPEN	0.00				-04-01-01	00-03-08	20.66		
	2	J3	6.00	05-10-08	03-00-12	2 X 4 2 X 4	01-03-08	01-02-00	37.42		
		JACK-OPEN	0.00				-02-01-01	00-03-08	23.34		
	2	J4	6.00	01-10-08	02-00-12	2 X 4 2 X 4	01-03-08	01-02-00	16.86		
		JACK-OPEN	0.00				-00-01-01	00-03-08	11.34		
	2	J5	6.00	01-10-08	03-00-12	2 X 4 2 X 4	01-03-08	01-02-00	21.96		
		JACK-OPEN	0.00				01-10-15	00-04-11	14.00		



Delivery Shiplist

DATE	08/24/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 282690 LOCATION: WATERDOWN
 BUILDER: GREENPARK - RUSSELL GARDENS PH 1 SUB-BUILDER:
 MODEL: ROSEWOOD 2 ELEVATION: 2

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	J7 JACK-OPEN	10.00 0.00	03-10-08	04-10-07	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 04-10-07	13.75 8.67		
	1	J8 JACK-OPEN	10.00 0.00	01-10-08	03-01-09	2 X 4	2 X 4	01-03-08 -00-01-01	01-07-11 00-03-08	9.99 7.00		
	1	J12 JACK-OPEN	10.00 0.00	03-10-08	03-01-09	2 X 4	2 X 4	01-03-08 -02-01-01	01-07-11 00-03-08	13.73 9.33		

TOTAL # TRUSS= 57.00 TOTAL BFT OF ALL TRUSSES= 2897.39 BFT. TOTAL WEIGHT OF ALL TRUSSES= 4629.51 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	LUS24-2	
3	Hangers	LUS24	
23	Hangers	LJS26DS	

TOTAL # ITEMS= 27.00



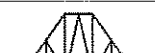
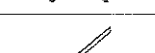
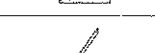
Delivery Shiplist

DATE	08/24/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 282695 LOCATION: WATERDOWN
 BUILDER: GREENPARK - RUSSELL GARDENS PH2 SUB-BUILDER:
 MODEL: ROSEWOOD 2 ELEVATION: 3

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1 2 Ply	T100 HIP GIRDER	10.00 0.00	31-07-00	04-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	365.50 218.68		
	1 2 Ply	T100Z HIP GIRDER	10.00 0.00	31-07-00	04-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	365.50 218.68		
	2	T2 HIP	10.00 0.00	31-07-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	260.78 168.34		
	2	T3 HIP	10.00 0.00	31-07-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	261.04 163.00		
	2	T4 HIP	10.00 0.00	31-07-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	284.48 177.66		
	2	T5 HIP	10.00 0.00	31-07-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	299.10 191.34		
	2	T6 HIP	10.00 0.00	31-07-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	303.02 188.34		
	2	T7 HIP	10.00 0.00	31-07-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	325.86 204.00		
	3	T8 HIP	10.00 0.00	31-07-00	11-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	505.26 312.99		
	1 3 Ply	T9Z HIP GIRDER	6.00 0.00	23-10-00	03-03-04	2 X 4	2 X 6	00-00-00 00-00-00	01-02-00 01-02-00	382.08 236.01		
	1	T30 HIP GIRDER	10.00 0.00	21-07-00	04-10-07	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	111.21 69.67		
	1	T31 HIP	10.00 0.00	21-07-00	06-06-07	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	96.54 60.33		
	3	T12 COMMON	10.00 0.00	12-08-00	06-11-00	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	182.28 118.50		
	2	T32 ROOF	10.00 5.00	12-08-00	06-11-00	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	130.48 88.00		
	1	T33 SCISSOR	10.00 5.00	08-02-00	05-00-08	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	39.70 26.83		
	21	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	352.59 224.07		
	3	J7 JACK-OPEN	10.00 0.00	03-10-08	04-10-07	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 04-10-07	41.25 26.01		
	1	J8 JACK-OPEN	10.00 0.00	01-10-08	03-01-09	2 X 4	2 X 4	00-00-00 -00-01-01	01-07-11 00-03-08	6.00 4.33		



Delivery Shiplist

DATE	08/24/17
SALES REP	Mario

JOB TRACK: 43954 LAYOUT ID: 282695 LOCATION: WATERDOWN
 BUILDER: GREENPARK - RUSSELL GARDENS PH2 SUB-BUILDER:
 MODEL: ROSEWOOD 2 ELEVATION: 3

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	13	J9 JACK-OPEN	10.00 0.00	01-11-08	03-03-04	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 03-03-04	113.36 78.00		
	1	J12 JACK-OPEN	10.00 0.00	03-10-08	03-01-09	2 X 4	2 X 4	01-03-08 -02-01-01	01-07-11 00-03-08	13.73 9.33		

TOTAL # TRUSS= 69.00

TOTAL BFT OF ALL TRUSSES=

2784.11 BFT. TOTAL WEIGHT OF ALL TRUSSES= 4439.76 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	HGUS26-2	
13	Hangers	LJS26DS	

TOTAL # ITEMS= 16.00

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 319.1 lbs FACTORED DOWN AT 2-11-8, 283.2 lbs FACTORED DOWN AT 28-7-8, 101.4 lbs FACTORED DOWN AT 5-0-4, 101.4 lbs FACTORED DOWN AT 7-0-4, 101.4 lbs FACTORED DOWN AT 9-0-4, 101.4 lbs FACTORED DOWN AT 11-0-4, 101.4 lbs FACTORED DOWN AT 13-0-4, 101.4 lbs FACTORED DOWN AT 15-0-4, 101.4 lbs FACTORED DOWN AT 18-6-12, 101.4 lbs FACTORED DOWN AT 18-6-12, 101.4 lbs FACTORED DOWN AT 20-6-12, 101.4 lbs FACTORED DOWN AT 22-6-12, AND 101.4 lbs FACTORED DOWN AT 24-6-12, AND 101.4 lbs FACTORED DOWN AT 26-6-12 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 1-0-4, 69.9 lbs FACTORED DOWN AT 3-0-4, 69.9 lbs FACTORED DOWN AT 5-0-4, 69.9 lbs FACTORED DOWN AT 7-0-4, 69.9 lbs FACTORED DOWN AT 9-0-4, 69.9 lbs FACTORED DOWN AT 11-0-4, 69.9 lbs FACTORED DOWN AT 13-0-4, 69.9 lbs FACTORED DOWN AT 15-0-4, 69.9 lbs FACTORED DOWN AT 16-6-12, 69.9 lbs FACTORED DOWN AT 18-6-12, 69.9 lbs FACTORED DOWN AT 20-6-12, 69.9 lbs FACTORED DOWN AT 22-6-12, 69.9 lbs FACTORED DOWN AT 24-6-12, 69.9 lbs FACTORED DOWN AT 26-6-12, AND 69.9 lbs FACTORED DOWN AT 28-6-12, AND 69.9 lbs FACTORED DOWN AT 30-6-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LOADING

TOTAL LOAD CASES: (4)

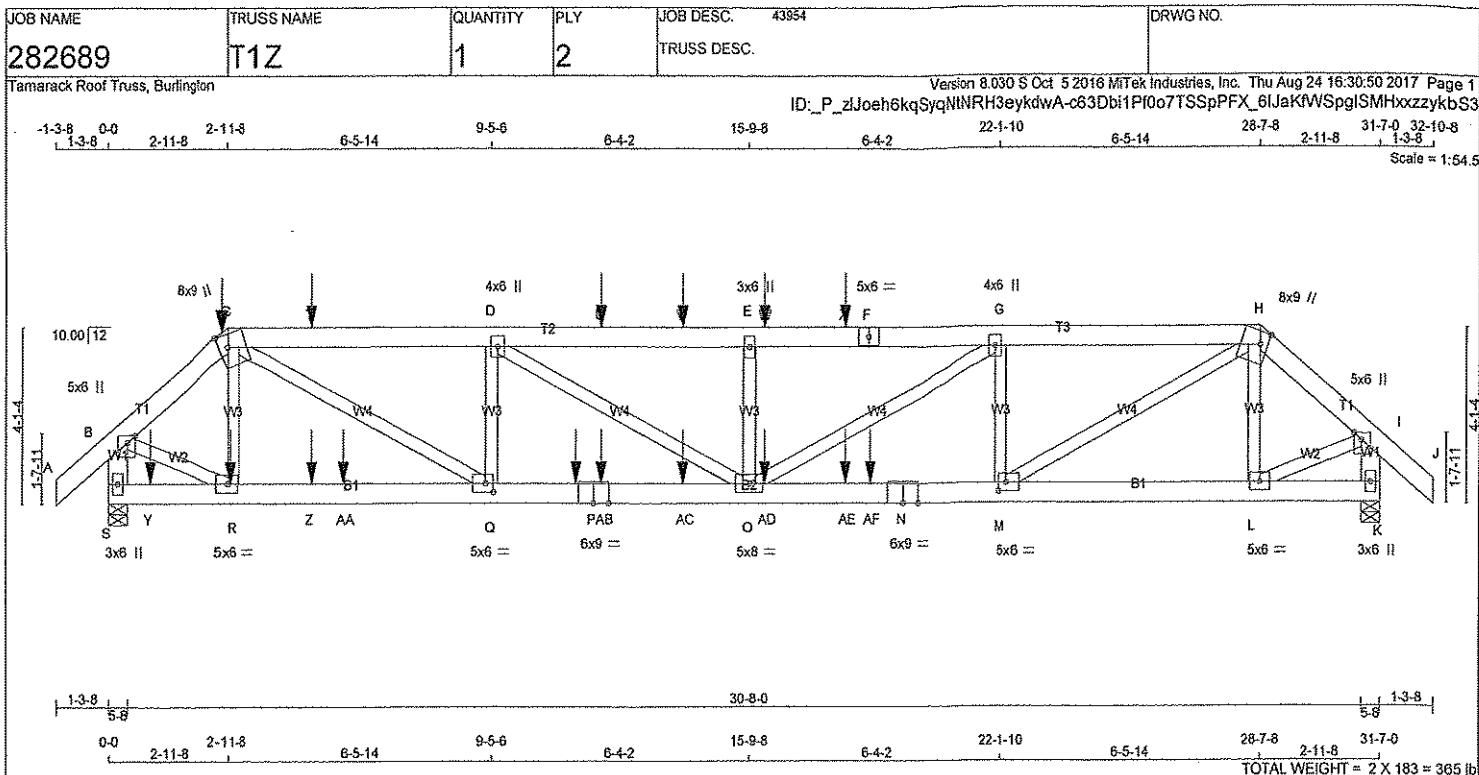
CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM	TO	LENGTH	FR-TO		
M-AO	0/2343	-28.0	-28.0	0.14 (1)	10.00		
AO-AP	0/2343	-28.0	-28.0	0.14 (1)	10.00		
AP-AQ	0/2343	-28.0	-28.0	0.14 (1)	10.00		
AQ-L	0/2343	-28.0	-28.0	0.14 (1)	10.00		
L-AR	0/0	-28.0	-28.0	0.06 (2)	10.00		
AR-K	0/0	-28.0	-28.0	0.06 (2)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-11-8	-16	-18	---	FRONT	VERT	DEAD
C	2-11-8	-141	-141	---	FRONT	VERT	TOTAL
C	2-11-8	-162	-162	---	FRONT	VERT	SNOW
F	18-6-12	-101	-101	---	FRONT	VERT	TOTAL
H	28-7-8	-16	-18	---	FRONT	VERT	DEAD
H	28-7-8	-105	-105	---	FRONT	VERT	TOTAL
H	28-7-8	-162	-162	---	FRONT	VERT	SNOW
L	28-6-12	-40	-70	---	FRONT	VERT	TOTAL
R	3-0-4	-40	-70	---	FRONT	VERT	TOTAL
T	5-0-4	-101	-101	---	FRONT	VERT	TOTAL
U	7-0-4	-101	-101	---	FRONT	VERT	TOTAL
V	9-0-4	-101	-101	---	FRONT	VERT	TOTAL
W	11-0-4	-101	-101	---	FRONT	VERT	TOTAL
X	13-0-4	-101	-101	---	FRONT	VERT	TOTAL
Y	15-0-4	-101	-101	---	FRONT	VERT	TOTAL
Z	16-6-12	-101	-101	---	FRONT	VERT	TOTAL
AA	20-6-12	-101	-101	---	FRONT	VERT	TOTAL
AB	22-6-12	-101	-101	---	FRONT	VERT	TOTAL
AC	24-6-12	-101	-101	---	FRONT	VERT	TOTAL
AD	26-6-12	-101	-101	---	FRONT	VERT	TOTAL
AE	1-0-4	-40	-70	---	FRONT	VERT	TOTAL
AF	5-0-4	-40	-70	---	FRONT	VERT	TOTAL
AG	7-0-4	-40	-70	---	FRONT	VERT	TOTAL
AH	9-0-4	-40	-70	---	FRONT	VERT	TOTAL
AJ	11-0-4	-40	-70	---	FRONT	VERT	TOTAL
AJ	13-0-4	-40	-70	---	FRONT	VERT	TOTAL
AK	15-0-4	-40	-70	---	FRONT	VERT	TOTAL
AL	16-6-12	-40	-70	---	FRONT	VERT	TOTAL
AM	18-6-12	-40	-70	---	FRONT	VERT	TOTAL
AN	20-6-12	-40	-70	---	FRONT	VERT	TOTAL
AO	22-6-12	-40	-70	---	FRONT	VERT	TOTAL
AP	24-6-12	-40	-70	---	FRONT	VERT	TOTAL
AQ	26-6-12	-40	-70	---	FRONT	VERT	TOTAL
AR	30-6-12	-40	-70	---	FRONT	VERT	TOTAL



DWG NO. TAM 42836-17
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x6	DRY	No.2 SPF
C - F	2x6	DRY	No.2 SPF
F - H	2x6	DRY	No.2 SPF
H - J	2x6	DRY	No.2 SPF
S - B	2x6	DRY	No.2 SPF
K - I	2x6	DRY	No.2 SPF
S - P	2x6	DRY	1650F 1.5E SPF
P - N	2x6	DRY	1650F 1.5E SPF
N - K	2x6	DRY	1650F 1.5E SPF

ALL WEBS 2x4 DRY No.2 SPF EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	2	12
C - F	2	12
F - H	2	12
H - J	2	12
S - B	2	12
K - I	2	12
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
S - P	2	12
P - N	2	12
N - K	2	12
WEBS : (0.122"x3") SPIRAL NAILS		
2x4	1	6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TTWW+m	MT20	8.0	9.0	3.50	2.25
D	TMVW+H	MT20	4.0	6.0		
E	TMW+w	MT20	3.0	6.0		
F	TS-I	MT20	5.0	6.0		
G	TMVW+H	MT20	4.0	6.0		
H	TTWW+m	MT20	8.0	9.0	3.50	2.25
I	TMVW+p	MT20	5.0	6.0	2.00	2.25
K	BMV1+p	MT20	3.0	6.0		
L	BMVW-I	MT20	5.0	6.0		
M	BMVW-I	MT20	5.0	6.0	2.50	2.25
N	BS-I	MT20	6.0	9.0		
O	BMVW+H	MT20	5.0	8.0		
P	BS-I	MT20	6.0	9.0		
Q	BMVW+H	MT20	5.0	6.0	2.50	2.25
R	BMVW-I	MT20	5.0	6.0		
S	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	UPLIFT	IN-SX
S	3671	0	0	5-8
K	3264	0	0	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX/MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
S	2998	1690 / 0	666 / 0
K	2643	1527 / 0	562 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.92 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 39	-84.3	-84.3 0.03 (1)	R-C	-507 / 163	0.04 (1)	
B-C	-3772 / 0	-84.3	-84.2 0.05 (1)	Q-D	0 / 4482	0.40 (1)	
C-T	-6777 / 0	-84.3	-84.3 0.25 (1)	L-D	-1825 / 0	0.16 (1)	
T-D	-6777 / 0	-84.3	-84.3 0.25 (1)	D-O	0 / 2254	0.20 (1)	
D-U	-8731 / 0	-84.3	-84.3 0.30 (1)	O-E	-785 / 0	0.07 (1)	
U-V	-8731 / 0	-84.3	-84.3 0.30 (1)	C-G	0 / 1954	0.17 (1)	
V-E	-8731 / 0	-84.3	-84.3 0.30 (1)	M-H	-1570 / 0	0.13 (1)	
E-W	-8731 / 0	-84.3	-84.3 0.27 (1)	M-G	0 / 5288	0.47 (1)	
W-X	-8731 / 0	-84.3	-84.3 0.27 (1)	L-H	-892 / 0	0.08 (1)	
X-F	-8731 / 0	-84.3	-84.3 0.27 (1)	B-R	0 / 3061	0.27 (1)	
F-G	-8731 / 0	-84.3	-84.3 0.27 (1)	L-I	0 / 2605	0.23 (1)	
G-H	-7037 / 0	-84.3	-84.3 0.22 (1)				
H-I	-3210 / 0	-84.3	-84.3 0.05 (1)				
I-J	0 / 39	-84.3	-84.3 0.03 (1)				
S-B	-3714 / 0	0.0	0.0 0.13 (1)				
K-I	-3197 / 0	0.0	0.0 0.12 (1)				
S-Y	0 / 0	-28.0	-28.0 0.09 (2)				
Y-R	0 / 0	-28.0	-28.0 0.09 (2)				
R-Z	0 / 2861	-28.0	-28.0 0.21 (1)				
Z-AA	0 / 2861	-28.0	-28.0 0.21 (1)				
AA-Q	0 / 2861	-28.0	-28.0 0.21 (1)				
Q-AB	0 / 6777	-28.0	-28.0 0.49 (1)				
AB-P	0 / 6777	-28.0	-28.0 0.49 (1)				
P-AC	0 / 6777	-28.0	-28.0 0.49 (1)				
AC-O	0 / 6777	-28.0	-28.0 0.49 (1)				
O-AD	0 / 7037	-28.0	-28.0 0.75 (1)				
AD-AE	0 / 7037	-28.0	-28.0 0.75 (1)				
AE-AF	0 / 7037	-28.0	-28.0 0.75 (1)				
AF-N	0 / 7037	-28.0	-28.0 0.75 (1)				
N-M	0 / 7037	-28.0	-28.0 0.75 (1)				
M-L	0 / 2416	-28.0	-28.0 0.27 (1)				
L-K	0 / 0	-28.0	-28.0 0.02 (1)				

8/5/77

S. KATSOULAKO

PROVINCE OF ONT.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE
C	2-11-8	-16	-18	-	FRONT	VERT	DEAD
C	2-11-8	-141	-141	-	BACK	VERT	TOTAL
C	2-11-8	-162	-162	-	FRONT	VERT	SNOW
P	12-2-0	-40	-70	-	BACK	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***

ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.05")
CALCULATED VERT. DEFL.(LL) = L/999 (0.25")
ALLOWABLE DEFL.(TL) = L/360 (1.05")
CALCULATED VERT. DEFL.(TL) = L/938 (0.40")

CSI: TC=0.30 (D-E:1), BC=0.75 (M-O:1),
WB=0.47 (H-M:1), SS=0.32 (M-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

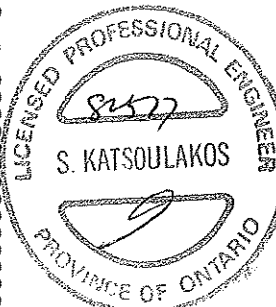
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	822	2284	1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (D) (INPUT = 0.90)
JSI METAL= 0.82 (P) (INPUT = 1.00)



DWG NO. TAM 42857-17
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME 282689	TRUSS NAME T1Z	QUANTITY 1	PLY 2	JOB DESC. 43954 TRUSS DESC.	DRWG NO.
--------------------	-------------------	---------------	----------	--------------------------------	----------

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MTek Industries, Inc. Thu Aug 24 16:30:50 2017 Page 2

ID: P_zJoeh6kgSyqNtNRH3eykdwA-c63Dbt1PI0o7TSSpPFX_6lJaKtWSpgiSMHxzykbS3

HANGERS NOTES

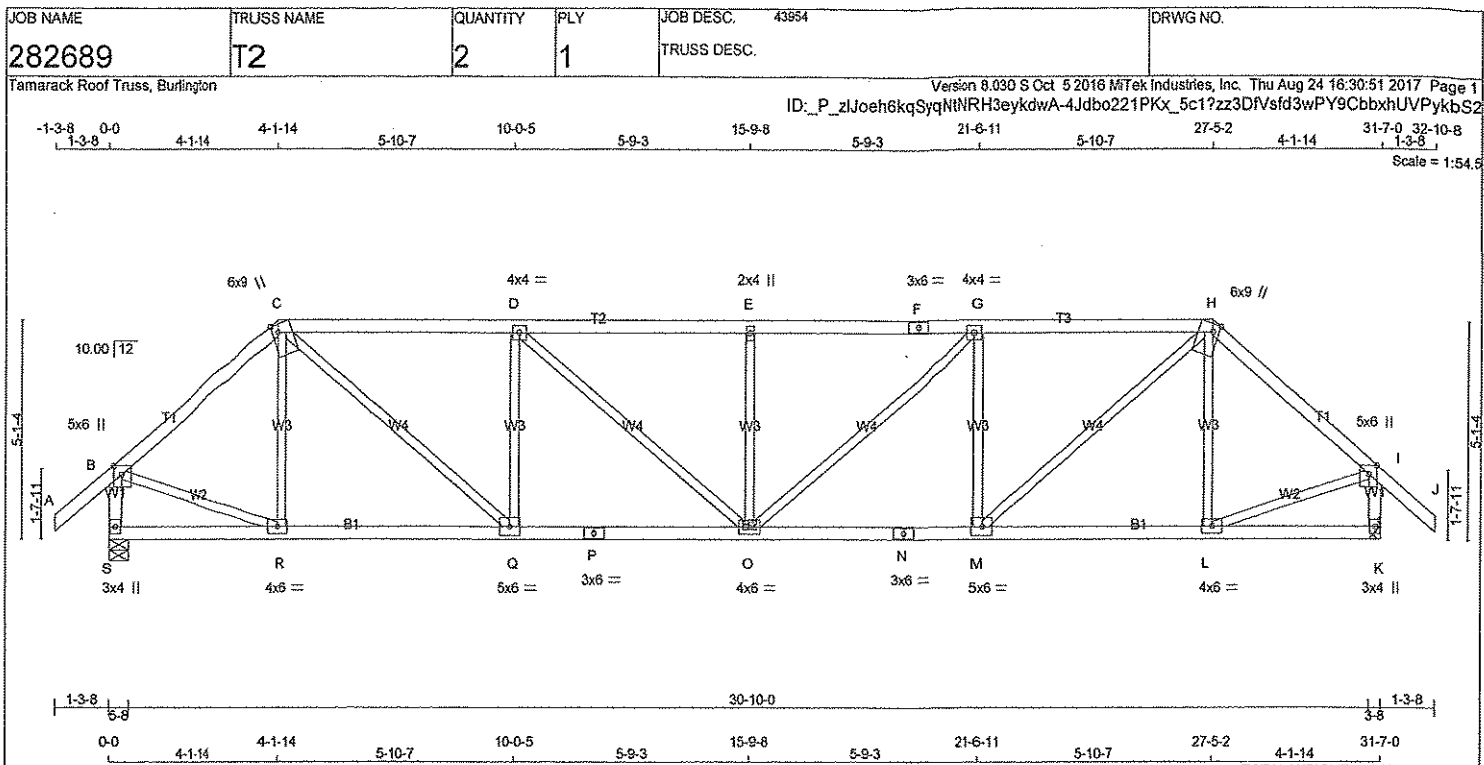
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 178.3 lbs FACTORED DOWN AT 2-11-8, 140.9 lbs FACTORED DOWN AT 2-11-8, 101.4 lbs FACTORED DOWN AT 5-0-4, 101.4 lbs FACTORED DOWN AT 12-2-0, 101.4 lbs FACTORED DOWN AT 14-2-0, AND 101.4 lbs FACTORED DOWN AT 16-2-0, AND 101.4 lbs FACTORED DOWN AT 18-2-0 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 1-0-4, 69.9 lbs FACTORED DOWN AT 3-0-4, 69.9 lbs FACTORED DOWN AT 5-0-4, 269.8 lbs FACTORED DOWN AT 5-9-8, 269.8 lbs FACTORED DOWN AT 11-6-8, 69.9 lbs FACTORED DOWN AT 12-2-0, 69.9 lbs FACTORED DOWN AT 14-2-0, 69.9 lbs FACTORED DOWN AT 16-2-0, AND 69.9 lbs FACTORED DOWN AT 18-2-0, AND 1501.9 lbs FACTORED DOWN AT 18-9-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
R	3-0-4	-40	-70	---	BACK	VERT	TOTAL
T	5-0-4	-101	-101	---	BACK	VERT	TOTAL
U	12-2-0	-101	-101	---	BACK	VERT	TOTAL
V	14-2-0	-101	-101	---	BACK	VERT	TOTAL
W	16-2-0	-101	-101	---	BACK	VERT	TOTAL
X	18-2-0	-101	-101	---	BACK	VERT	TOTAL
Y	1-0-4	-40	-70	---	BACK	VERT	TOTAL
Z	5-0-4	-40	-70	---	BACK	VERT	TOTAL
AA	5-9-8	-270	-270	---	BACK	VERT	TOTAL
AB	11-6-8	-270	-270	---	BACK	VERT	TOTAL
AC	14-2-0	-40	-70	---	BACK	VERT	TOTAL
AD	16-2-0	-40	-70	---	BACK	VERT	TOTAL
AE	18-2-0	-40	-70	---	BACK	VERT	TOTAL
AF	18-9-8	-1502	-1502	---	BACK	VERT	TOTAL



DWG NO. TAM 42057-17
STRUCTURAL
COMPONENT ONLY



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2 SPF
C - F	2x4	DRY	No.2 SPF
F - H	2x4	DRY	No.2 SPF
H - J	2x4	DRY	No.2 SPF
S - B	2x4	DRY	No.2 SPF
K - I	2x4	DRY	No.2 SPF
S - P	2x4	DRY	No.2 SPF
P - N	2x4	DRY	No.2 SPF
N - K	2x4	DRY	No.2 SPF
ALL WEBS EXCEPT	2x3	DRY	No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.50	2.25
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW+m	MT20	6.0	9.0	Edge	1.75
I	TMVW+p	MT20	5.0	6.0	2.50	2.25
K	BMV1+p	MT20	3.0	4.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMWW-t	MT20	5.0	6.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	6.0		
P	BS-t	MT20	3.0	6.0		
Q	BMWW-t	MT20	5.0	6.0		
R	BMWW-t	MT20	4.0	6.0		
S	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REORD BRG
	VERT	HORZ	DOWN	HORZ		
S	1889	0	1889	0	5-8	5-8
K	1889	0	1889	0	HANGER BY OTHERS	MIN. SEAT SIZE: 3-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
S	1535	879 / 0	332 / 0	0 / 0	0 / 0	324 / 0	0 / 0
K	1535	879 / 0	332 / 0	0 / 0	0 / 0	324 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.47 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED		MAX. UNBRAC	MEMB.	MAX. FACTORED			
		VERT. LOAD (PLF)	LC1 (LC)			FORCE (LBS)	MAX CSI (LC)		
FR-TO		FROM	TO	LENGTH	FR-TO				
A-B	0 / 37	-84.3	-84.3	0.12 (1)	10.00	R-C	-258 / 59 0.10 (1)		
B-C	-1772 / 0	-84.3	-84.3	0.32 (1)	4.69	C-Q	0 / 1605 0.36 (1)		
C-D	-2595 / 0	-84.3	-84.3	0.60 (1)	3.71	Q-D	-853 / 0 0.33 (1)		
D-E	-2959 / 0	-84.3	-84.3	0.65 (1)	3.47	D-O	0 / 474 0.11 (1)		
E-F	-2959 / 0	-84.3	-84.3	0.65 (1)	3.47	O-E	-448 / 0 0.17 (1)		
F-G	-2959 / 0	-84.3	-84.3	0.65 (1)	3.47	G-O	0 / 474 0.11 (1)		
G-H	-2595 / 0	-84.3	-84.3	0.60 (1)	3.71	M-G	-853 / 0 0.33 (1)		
H-I	-1772 / 0	-84.3	-84.3	0.32 (1)	4.69	M-H	0 / 1605 0.36 (1)		
I-J	0 / 37	-84.3	-84.3	0.12 (1)	10.00	L-H	-258 / 59 0.10 (1)		
S-B	-1847 / 0	0.0	0.0	0.20 (1)	6.16	B-R	0 / 1426 0.32 (1)		
K-I	-1847 / 0	0.0	0.0	0.20 (1)	6.16	L-I	0 / 1426 0.32 (1)		
S-R	0 / 0	-28.0	-28.0	0.17 (2)	10.00				
R-Q	0 / 1352	-28.0	-28.0	0.34 (2)	10.00				
Q-P	0 / 2595	-28.0	-28.0	0.51 (1)	10.00				
P-O	0 / 2595	-28.0	-28.0	0.51 (1)	10.00				
O-N	0 / 2595	-28.0	-28.0	0.51 (1)	10.00				
N-M	0 / 2595	-28.0	-28.0	0.51 (1)	10.00				
M-L	0 / 1352	-28.0	-28.0	0.34 (2)	10.00				
L-K	0 / 0	-28.0	-28.0	0.17 (2)	10.00				

PROFESSIONAL

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.05")
CALCULATED VERT. DEFL.(LL) = L/999 (0.17")
ALLOWABLE DEFL.(TL) = L/360 (1.05")
CALCULATED VERT. DEFL.(TL) = L/999 (0.28")

CSI: TC=0.65 (E-G:1), BC=0.51 (M-O:1), WB=0.36 (C-Q:1), SSI=0.23 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

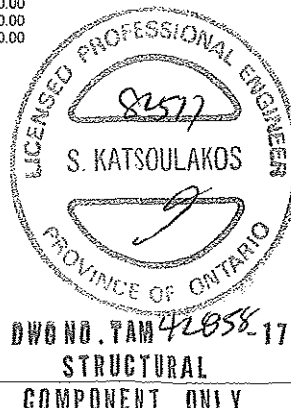
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (L) (INPUT = 0.90)
JSI METAL= 0.73 (P) (INPUT = 1.00)

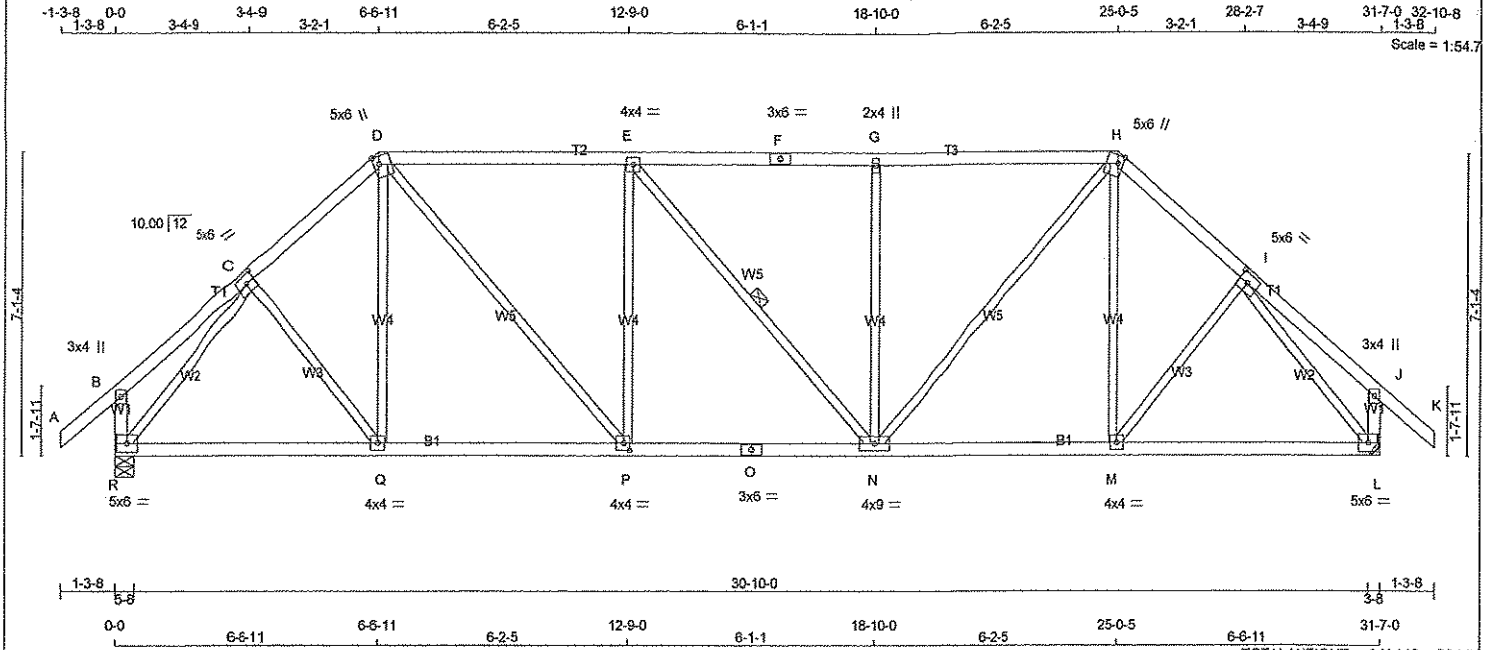


DWG NO. TAM 42859-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43854	DRWG NO.
282689	T4	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 Mittek Industries, Inc. Thu Aug 24 16:30:51 2017 Page 1
ID: P_zJcoeh6kqSyqNtNRH3eykdwA-4Jdbo221PKx_5c17zz3DFVsho3wFY17bbxhUVPykbS2



TOTAL WEIGHT = 2 X 142 = 284 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
R - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - L	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
DRY: SEASONED LUMBER			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	DOWN	IN-SX	IN-SX
R 1889	0	1889	0
L 1889	0	1889	0
HANGER BY OTHERS			
MIN. SEAT SIZE: 3-8			

UNFACTORED REACTIONS

1ST LCASE	MAX	MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
R	1535	879 / 0	332 / 0
L	1535	879 / 0	332 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.13 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS			
MEMB.	MAX. FACTORED	FACTORED	MAX.
	FORCE	VERT. LOAD	MAX.
	(LBS)	(PLF)	CS (L/C)
FR-TO		FROM TO	LENGTH
A-B	0 / 37	-84.3 -84.3	0.12 (1)
B-C	0 / 18	-84.3 -84.3	0.13 (1)
C-D	-1811 / 0	-84.3 -84.3	0.17 (1)
D-E	-2034 / 0	-84.3 -84.3	0.57 (1)
E-F	-2033 / 0	-84.3 -84.3	0.57 (1)
F-G	-2033 / 0	-84.3 -84.3	0.57 (1)
G-H	-2033 / 0	-84.3 -84.3	0.57 (1)
H-I	-1812 / 0	-84.3 -84.3	0.17 (1)
I-J	0 / 18	-84.3 -84.3	0.13 (1)
J-K	0 / 37	-84.3 -84.3	0.12 (1)
R-B	-226 / 0	0.0 0.0	0.02 (1)
L-J	-226 / 0	0.0 0.0	0.02 (1)
R-Q	0 / 1286	-28.0 -28.0	0.43 (2)
Q-P	0 / 1375	-28.0 -28.0	0.45 (2)
P-O	0 / 2035	-28.0 -28.0	0.43 (1)
O-N	0 / 2035	-28.0 -28.0	0.43 (1)
N-M	0 / 1375	-28.0 -28.0	0.45 (2)
M-L	0 / 1286	-28.0 -28.0	0.44 (2)

WEBS			
MEMB.	MAX. FACTORED	MAX.	MAX.
	FORCE	UNBRAC	CS (L/C)
	(LBS)		
C-Q	0 / 178	0.04 (2)	
Q-D	0 / 231	0.05 (3)	
D-P	0 / 979	0.22 (1)	
P-E	-559 / 0	0.49 (1)	
E-N	-2 / 0	0.00 (1)	
N-G	-559 / 0	0.49 (1)	
G-H	0 / 977	0.22 (1)	
H-I	0 / 232	0.05 (3)	
M-I	0 / 178	0.04 (2)	
R-C	-2043 / 0	0.89 (1)	
I-L	-2044 / 0	0.89 (1)	

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F.
RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED
ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.05")
CALCULATED VERT. DEFL.(LL) = L/999 (0.12")
ALLOWABLE DEFL.(TL) = L/360 (1.05")
CALCULATED VERT. DEFL.(TL) = L/999 (0.19")

CSI: TC=0.57 (D-E-1), BC=0.45 (M-N-2), WB=0.88
(I-L-1), SSI=0.24 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN
THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.64 (O) (INPUT = 1.00)



JOB NAME 282689	TRUSS NAME T5	QUANTITY 2	PLY 1	JOB DESC. 43954 TRUSS DESC.	DRWG NO.
---------------------------	-------------------------	----------------------	-----------------	-----------------------------------	----------

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016

MITek Industries, Inc.

Thu Aug 24 16:30:52 2017

Page 1

ID_P_zlJoah6kqSygNINRH3eykdwa-YVA_003fAd3rjmcBxgaSBjOuCTFFHYVlpbQ22rykbs1

5-1-3-8

0-0

3-11-13

3-11-13

3-9-5

7-9-1

5-4-11

13-1-12

5-3-7

18-5-4

5-4-11

23-9-15

3-9-5

27-7-3

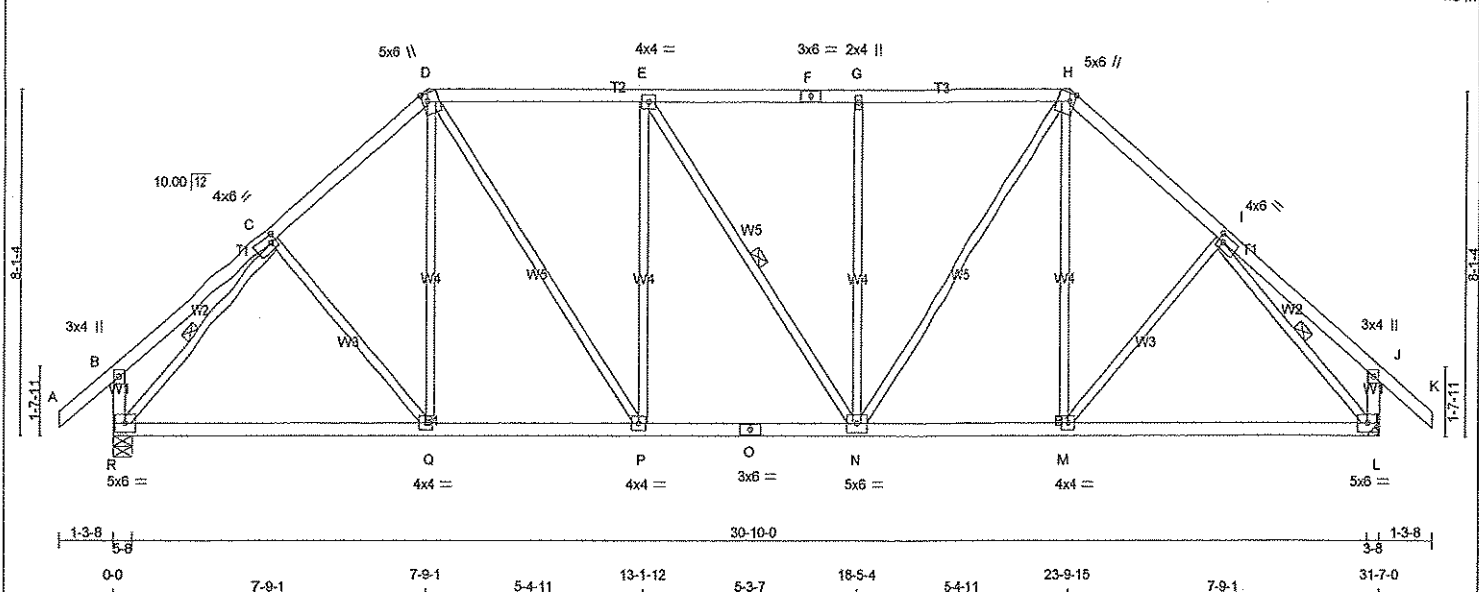
3-11-13

31-7-0

32-10-8

1-3-8

Scale = 1:54.7



TOTAL WEIGHT = 2 X 150 = 299 lb

LUMBER

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2 SPF
D - F	2x4	DRY	No.2 SPF
F - H	2x4	DRY	No.2 SPF
H - K	2x4	DRY	No.2 SPF
R - B	2x4	DRY	No.2 SPF
L - J	2x4	DRY	No.2 SPF
R - O	2x4	DRY	No.2 SPF
O - L	2x4	DRY	No.2 SPF

ALL WEBS	2x3	DRY	No.2 SPF
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	3.0	4.0		
C TMWW-t	MT20	4.0	6.0	2.00	1.50
D TTWW+m	MT20	5.0	6.0	2.25	1.50
E TMWW-t	MT20	4.0	4.0		
F TS-t	MT20	3.0	6.0		
G TMW+w	MT20	2.0	4.0		
H TTWW+m	MT20	5.0	6.0	2.25	1.50
I TMWW-t	MT20	4.0	6.0	2.00	1.50
J TMV+p	MT20	3.0	4.0		
L BMVW1-t	MT20	5.0	6.0		
M, P, Q					
M BMWW-t	MT20	4.0	4.0		
N BMWW-t	MT20	5.0	6.0		
O BS-t	MT20	3.0	6.0		
R BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQRD
JT	VERT	DOWN	HORZ	UPLIFT
R	1889	0	1889	0
L	1889	0	1889	0

HANGER BY OTHERS
MIN. SEAT SIZE: 3-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1535	879 / 0	332 / 0	0 / 0	0 / 0	324 / 0	0 / 0
L	1535	879 / 0	332 / 0	0 / 0	0 / 0	324 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N, C-R, I-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN
THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. FACTORED (PLF) CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED (PLF) CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 37	-84.3	-84.3 0.12 (1)	10.00	C-Q	0 / 133	0.03 (3)
B-C	0 / 23	-84.3	-84.3 0.19 (1)	10.00	Q-D	0 / 304	0.07 (2)
C-D	-1784 / 0	-84.3	-84.3 0.25 (1)	4.77	D-P	0 / 746	0.17 (1)
D-E	-1777 / 0	-84.3	-84.3 0.40 (1)	4.58	P-E	-485 / 0	0.61 (1)
E-F	-1776 / 0	-84.3	-84.3 0.40 (1)	4.58	E-N	-2 / 0	0.00 (1)
F-G	-1776 / 0	-84.3	-84.3 0.40 (1)	4.58	N-G	-484 / 0	0.61 (1)
G-H	-1775 / 0	-84.3	-84.3 0.39 (1)	4.59	N-H	0 / 744	0.17 (1)
H-I	-1784 / 0	-84.3	-84.3 0.25 (1)	4.77	M-H	0 / 304	0.07 (2)
I-J	0 / 23	-84.3	-84.3 0.19 (1)	10.00	M-I	0 / 133	0.03 (3)
J-K	0 / 37	-84.3	-84.3 0.12 (1)	10.00	R-C	-2045 / 0	0.51 (1)
R-B	-243 / 0	0.0	0.0 0.03 (1)	7.81	I-L	-2045 / 0	0.51 (1)
L-J	-243 / 0	0.0	0.0 0.03 (1)	7.81			
R-Q	0 / 1327	-28.0	-28.0 0.52 (2)	10.00			
Q-P	0 / 1352	-28.0	-28.0 0.53 (2)	10.00			
P-O	0 / 1777	-28.0	-28.0 0.36 (1)	10.00			
O-N	0 / 1777	-28.0	-28.0 0.36 (1)	10.00			
N-M	0 / 1352	-28.0	-28.0 0.53 (2)	10.00			
M-L	0 / 1327	-28.0	-28.0 0.52 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F.
RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED
ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.05")
CALCULATED VERT. DEFL.(LL) = L/999 (0.16")
ALLOWABLE DEFL.(TL) = L/360 (1.05")
CALCULATED VERT. DEFL.(TL) = L/999 (0.27")

CSI: TC=0.40 (D-E:1), BC=0.53 (M-N:2), WB=0.61
(E-P:1), SSI=0.21 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN
THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	618	354	1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

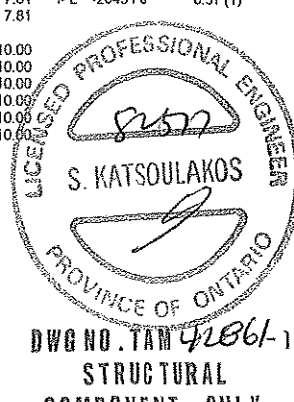
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (I) (INPUT = 0.90)
JSI METAL= 0.57 (O) (INPUT = 1.00)

DWG NO. TAM 42861-17

STRUCTURAL

COMPONENT - ONLY

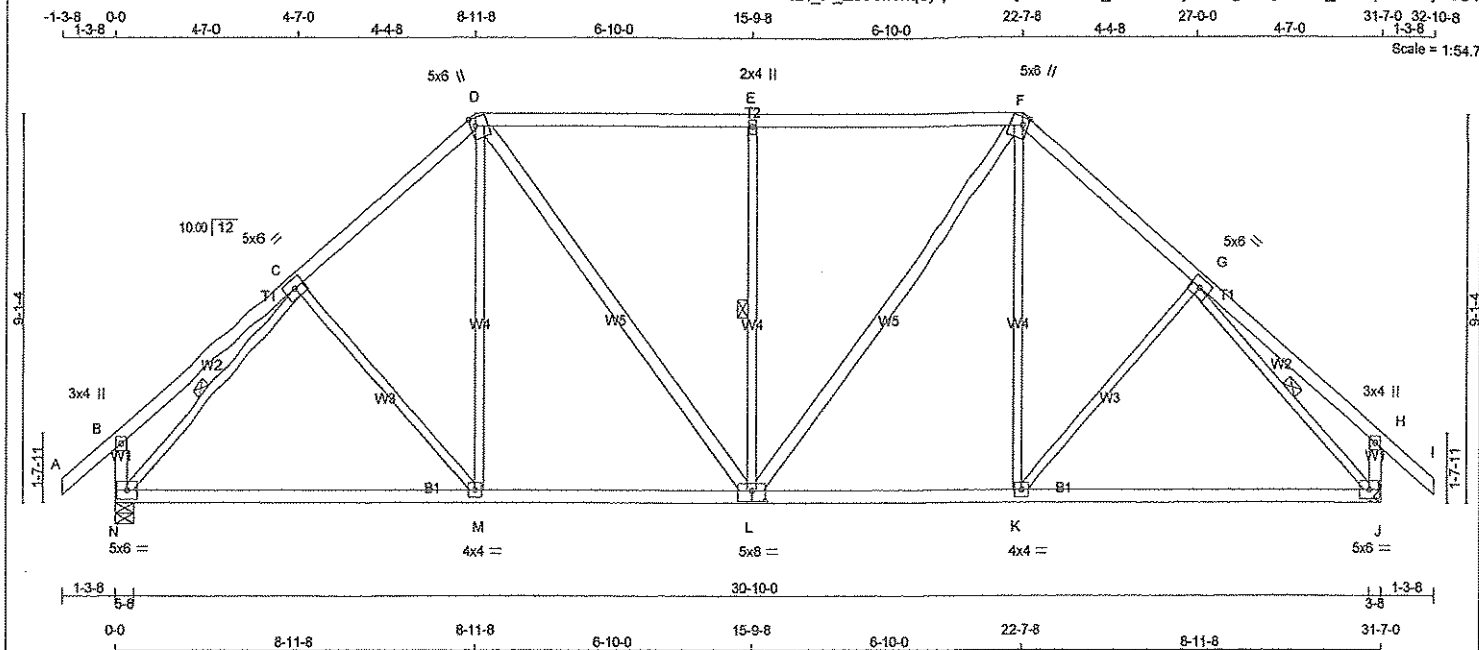


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
282689	T6	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 M/Tek Industries, Inc. Thu Aug 24 16:30:52 2017 Page 1

ID: P_zJoeh6kqSyqNWRH3eykdwA-YVA_0O3fAd3jmcBXgaSBjOsiTD_HYRlpbQ22rykbS1



TOTAL WEIGHT = 2 X 152 = 303 lb

LUMBER	SIZE	DESCR.
N. L. G. A. RULES		
CHORDS		
A - D	2x4 DRY	No.2 SPF
D - F	2x4 DRY	No.2 SPF
F - I	2x4 DRY	No.2 SPF
N - B	2x4 DRY	No.2 SPF
J - H	2x4 DRY	No.2 SPF
N - L	2x4 DRY	No.2 SPF
L - J	2x4 DRY	No.2 SPF

ALL WEBS	SIZE	DESCR.
EXCEPT		
D - L	2x4 DRY	No.2 SPF
L - F	2x4 DRY	No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)	TYPE	PLATES	W	LEN	Y	X
JT	TMV+p	MT20	3.0	4.0		
B	TMV+p	MT20	5.0	6.0		
C	TMV+p	MT20	5.0	6.0		
D	TMV+p	MT20	5.0	6.0	2.25	1.50
E	TMV+p	MT20	5.0	6.0		
F	TMV+p	MT20	5.0	6.0	2.25	1.50
G	TMV+p	MT20	5.0	6.0		
H	TMV+p	MT20	5.0	6.0		
J	TMV+p	MT20	5.0	6.0		
K	TMV+p	MT20	5.0	6.0		
L	TMV+p	MT20	5.0	6.0	3.00	4.00
M	TMV+p	MT20	5.0	6.0		
N	TMV+p	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	IN-SX	IN-SX
N	1889	0	5-8	5-8
J	1889	0	HANGER BY OTHERS	
			MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SC
N	1535	879 / 0	332 / 0	0 / 0	0 / 0	324 / 0	0
J	1535	879 / 0	332 / 0	0 / 0	0 / 0	324 / 0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.47 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-L, C-N, G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MEMB.	FORCE (LBS)
FR-TO			FR-TO	
A-B	0 / 37	-84.3 -84.3 0.12 (1)	C-M	-56 / 108
B-C	0 / 28	-84.3 -84.3 0.27 (1)	M-D	0 / 421
C-D	-1745 / 0	-84.3 -84.3 0.25 (1)	D-L	0 / 535
D-E	-1648 / 0	-84.3 -84.3 0.56 (1)	E-L	-706 / 0
E-F	-1648 / 0	-84.3 -84.3 0.56 (1)	L-F	0 / 535
F-G	-1745 / 0	-84.3 -84.3 0.25 (1)	K-F	0 / 421
G-H	0 / 28	-84.3 -84.3 0.27 (1)	K-G	-56 / 108
H-I	0 / 37	-84.3 -84.3 0.12 (1)	N-C	-2039 / 0
N-B	-261 / 0	0.0 0.0 0.03 (1)	G-J	-2039 / 0
J-H	-261 / 0	0.0 0.0 0.03 (1)		
N-M	0 / 1354	-28.0 -28.0 0.67 (2)		
M-L	0 / 1320	-28.0 -28.0 0.68 (2)		
L-K	0 / 1320	-28.0 -28.0 0.68 (2)		
K-J	0 / 1354	-28.0 -28.0 0.67 (2)		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
DL = 3.0 PSF	
BOT CH.	LL = 10.5 PSF
DL = 7.0 PSF	
TOTAL LOAD = 46.1 PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.05")
CALCULATED VERT. DEFL.(LL) = L/999 (0.26")
ALLOWABLE DEFL.(TL) = L/360 (1.05")
CALCULATED VERT. DEFL.(TL) = L/869 (0.44")

CSI: TC=0.56 (D-E:1), BC=0.68 (K-L:2), WB=0.62 (G-J:1), SSI=0.28 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

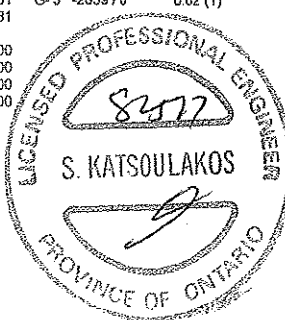
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(PRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

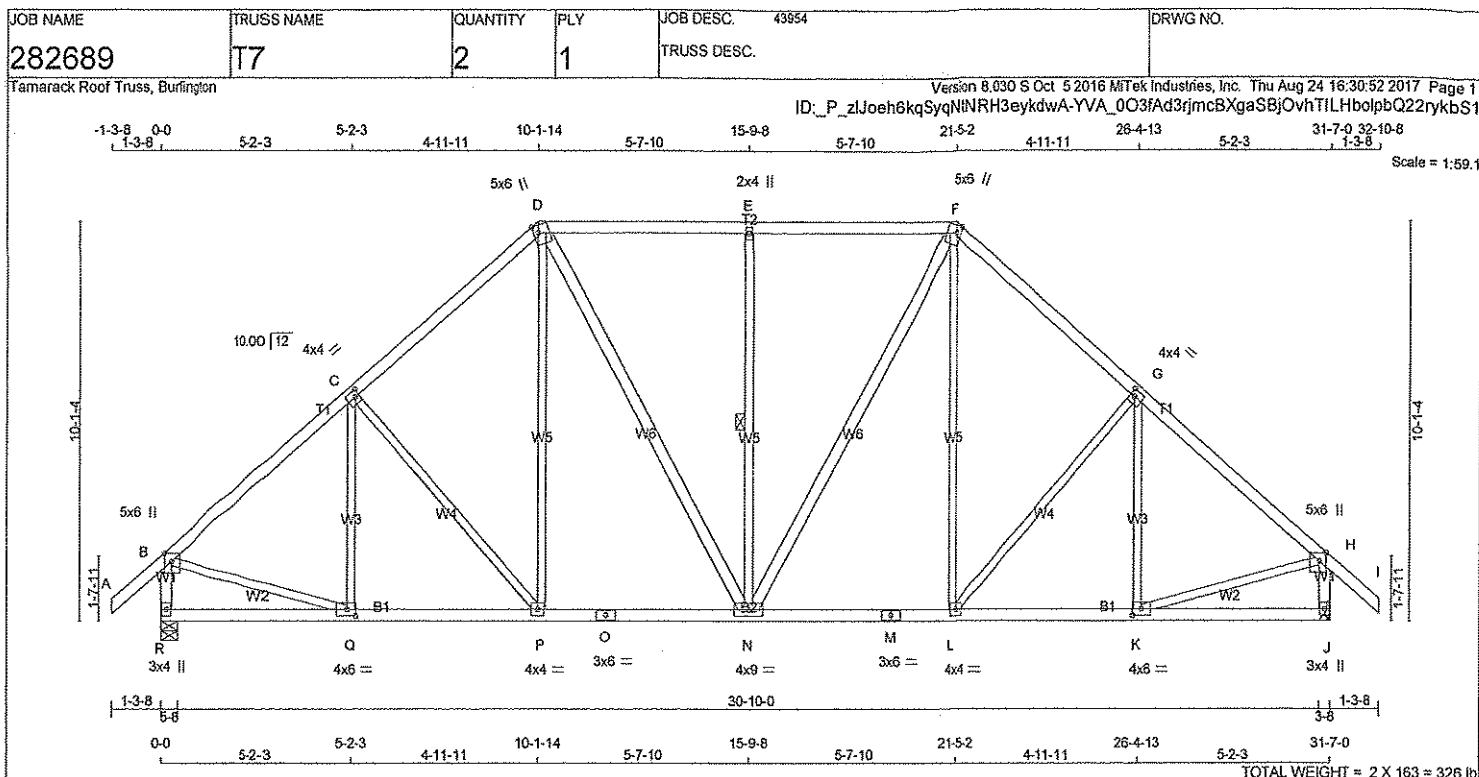
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (G) (INPUT = 0.90)
JSI METAL= 0.50 (G) (INPUT = 1.00)



DWG NO. TAM 4266-17
STRUCTURAL
COMPONENT ONLY



LUMBER
 N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.50	2.25
C	TMVW-l	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMVW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMVW-l	MT20	4.0	4.0	2.00	1.25
H	TMVW+p	MT20	5.0	6.0	2.50	2.25
J	BMV1+p	MT20	3.0	4.0		
K	BMVW-l	MT20	4.0	6.0	2.00	2.75
L	BMVW-l	MT20	4.0	4.0		
M	BS-l	MT20	3.0	6.0		
N	BMVW+l	MT20	4.0	9.0		
O	BS-l	MT20	3.0	6.0		
P	BMVW-l	MT20	4.0	4.0		
Q	BMVW-l	MT20	4.0	6.0	2.00	2.75
R	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	DOWN	IN-SX	IN-SX
R	1889	0	5-8	5-8
J	1889	0	5-8	5-8

HANGER BY OTHERS
 MIN. SEAT SIZE: 3-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1535	879 / 0	332 / 0	0 / 0	0 / 0	324 / 0	0 / 0
J	1535	879 / 0	332 / 0	0 / 0	0 / 0	324 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.64 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)
FR-TO					FR-TO		
A-B	0 / 37	-84.3	-84.3	0.12 (1)	10.00	Q-C	-202 / 101
B-C	-1820 / 0	-84.3	-84.3	0.35 (1)	4.64	C-P	-257 / 0
C-D	-1665 / 0	-84.3	-84.3	0.33 (1)	4.83	P-D	0 / 381
D-E	-1466 / 0	-84.3	-84.3	0.37 (1)	4.99	D-N	0 / 425
E-F	-1466 / 0	-84.3	-84.3	0.37 (1)	4.99	N-E	-578 / 0
F-G	-1665 / 0	-84.3	-84.3	0.33 (1)	4.83	N-F	0 / 425
G-H	-1820 / 0	-84.3	-84.3	0.35 (1)	4.64	L-F	0 / 381
H-I	0 / 37	-84.3	-84.3	0.12 (1)	10.00	L-G	-257 / 0
R-B	-1829 / 0	0.0	0.0	0.19 (1)	6.19	K-G	-202 / 101
J-H	-1829 / 0	0.0	0.0	0.19 (1)	6.19	B-Q	0 / 1467
						K-H	0 / 1467
R-Q	0 / 0	-28.0	-28.0	0.17 (3)	10.00		
Q-P	0 / 1423	-28.0	-28.0	0.34 (2)	10.00		
P-O	0 / 1255	-28.0	-28.0	0.33 (2)	10.00		
O-N	0 / 1255	-28.0	-28.0	0.33 (2)	10.00		
N-M	0 / 1255	-28.0	-28.0	0.33 (2)	10.00		
M-L	0 / 1255	-28.0	-28.0	0.33 (2)	10.00		
L-K	0 / 1423	-28.0	-28.0	0.34 (2)	10.00		
K-J	0 / 0	-28.0	-28.0	0.17 (3)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.05")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
 ALLOWABLE DEFL.(TL)= L/360 (1.05")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.12")

CSI: TC=0.37 (D-E:1), BC=0.34 (K-L:2), WB=0.40 (E-N:1), SSI=0.23 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

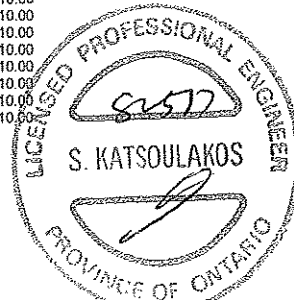
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (Q) (INPUT = 0.90)
 JSI METAL= 0.37 (Q) (INPUT = 1.00)



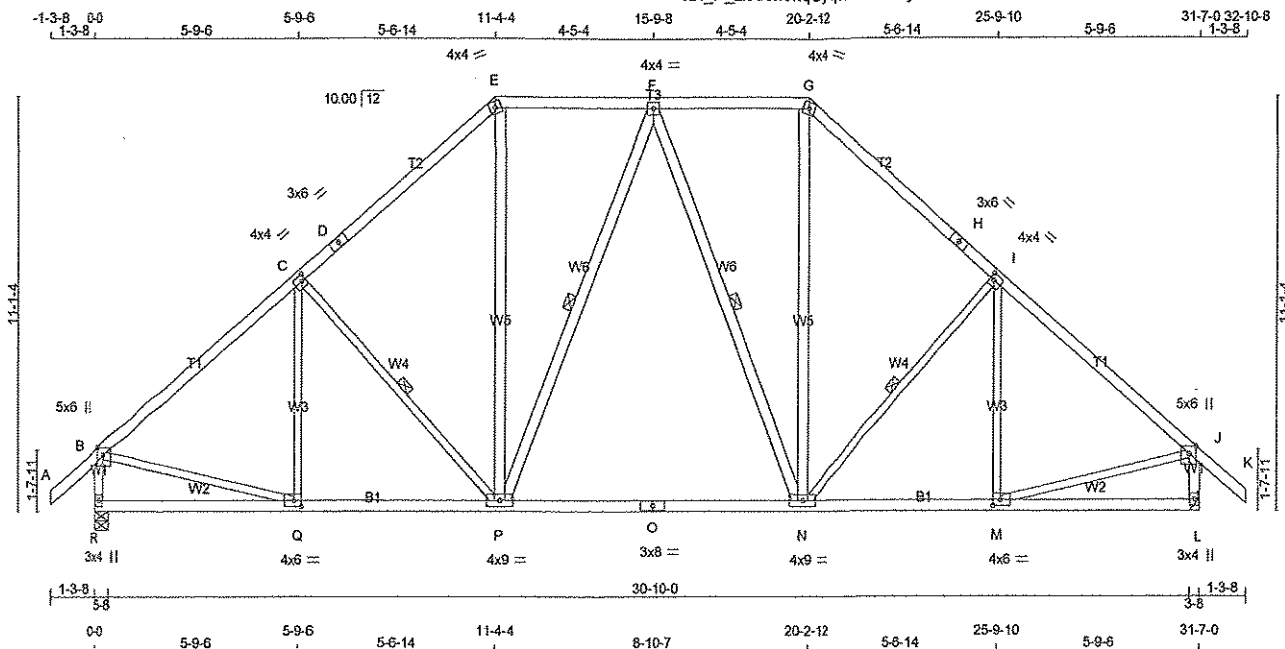
DRG NO. TAM 42863-17
 STRUCTURAL
 COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
282689	T8	3	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 Mitek Industries, Inc. Thu Aug 24 16:30:53 2017 Page 1
ID: P_zJ0eh6kqSyqNtNRH3eykdwA-0hkMDK4HxxBikWBO5O5hkwx3ltcx03Au2FAbalykbSO

Scale = 1:62.5



TOTAL WEIGHT = 3 X 168 = 505 lb

LUMBER				DESCR.	
N. L. G. A. RULES					
CHORDS	SIZE	LUMBER			
A - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
G - H	2x4	DRY	No.2	SPF	
H - K	2x4	DRY	No.2	SPF	
R - B	2x4	DRY	No.2	SPF	
L - J	2x4	DRY	No.2	SPF	
R - O	2x4	DRY	No.2	SPF	
O - L	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
P - E	2x4	DRY	No.2	SPF	
F - F	2x4	DRY	No.2	SPF	
F - N	2x4	DRY	No.2	SPF	
N - G	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.50	2.25
C	TMVW-i	MT20	4.0	4.0	2.00	1.25
D	TS-i	MT20	3.0	6.0		
E	TTW-m	MT20	4.0	4.0		
F	TMVW-i	MT20	4.0	4.0		
G	TTW-m	MT20	4.0	4.0		
H	TS-i	MT20	3.0	6.0		
I	TMVW-i	MT20	4.0	4.0	2.00	1.25
J	TMVW+p	MT20	5.0	6.0	2.50	2.25
L	BMV1+p	MT20	3.0	4.0		
M	BMVW-i	MT20	4.0	6.0	2.00	2.50
N	BMVW-i	MT20	4.0	9.0		
O	BS-i	MT20	3.0	8.0		
P	BMVW-i	MT20	4.0	9.0		
Q	BMVW-i	MT20	4.0	6.0	2.00	2.50
R	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	DOWN	HORZ	UPLIFT	IN-SX
R	1889	0	1889	0	5-8
L	1889	0	1889	0	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
R	1535	879 / 0	332 / 0	0 / 0	0 / 0	324 / 0	0 / 0
L	1535	879 / 0	332 / 0	0 / 0	0 / 0	324 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.52 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-P, F-P, F-N, I-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 37	Q-C	-163 / 124
B-C	-1828 / 0	C-P	-336 / 0
C-D	-1607 / 0	P-E	0 / 683
D-E	-1607 / 0	F-N	-241 / 0
E-F	-1212 / 0	F-N	-241 / 0
F-G	-1212 / 0	N-G	0 / 683
G-H	-1607 / 0	N-I	-336 / 0
H-I	-1607 / 0	M-I	-163 / 124
I-J	-1828 / 0	B-Q	0 / 1468
J-K	0 / 37	M-J	0 / 1468
R-B	-1820 / 0		
L-J	-1820 / 0		
R-Q	0 / 0		
Q-P	0 / 1432		
P-O	0 / 1302		
O-N	0 / 1302		
N-M	0 / 1432		
M-L	0 / 0		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.05")
CALCULATED VERT. DEFL.(LL) = L/999 (0.17")
ALLOWABLE DEFL.(TL) = L/360 (1.05")
CALCULATED VERT. DEFL.(TL) = L/999 (0.28")

CSI: TC=0.44 (I-J:1), BC=0.50 (M-N:2), WB=0.33 (B-Q:1), SSI=0.18 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

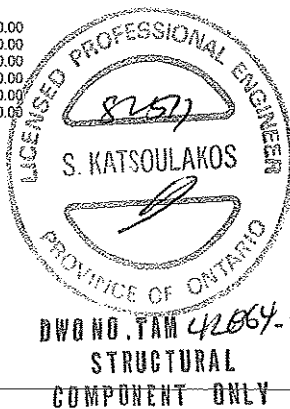
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (J) (INPUT = 0.90)
JSI METAL= 0.70 (O) (INPUT = 1.00)

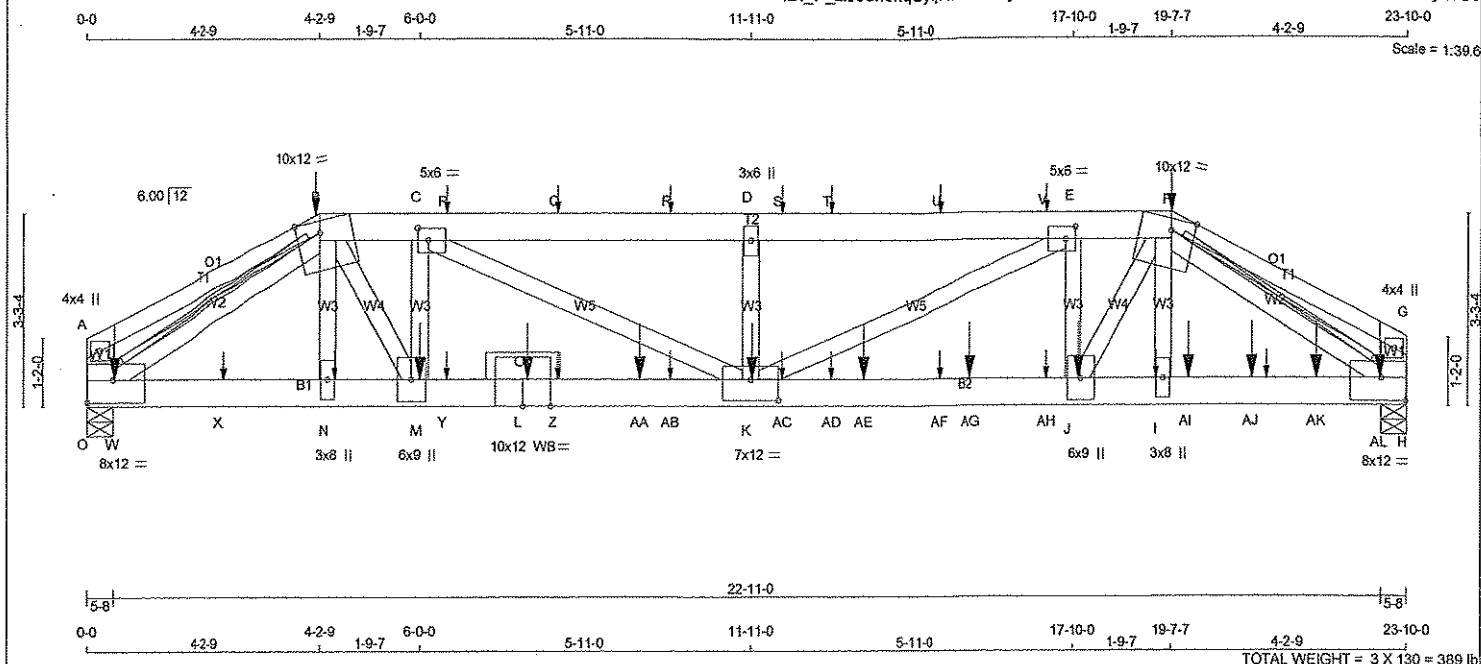


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
282689	T9	1	3	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 M/Tek Industries, Inc. Thu Aug 24 16:30:53 2017 Page 1

ID: P_zJoeh6kqSygNINRH3eykdwA-0hkMDK4HxxBiKwBO50hkw0tWe0x3u2FABalykbSO



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2 SPF
B - F	2x4	DRY	No.2 SPF
F - G	2x4	DRY	No.2 SPF
O - A	2x6	DRY	No.2 SPF
H - G	2x6	DRY	No.2 SPF
O - L	2x6	DRY	1650F 1.5E SPF
L - H	2x6	DRY	1650F 1.5E SPF

ALL WEBS 2x4 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - B	12	SIDE(91.5)
F - G	12	SIDE(91.5)
B - F	2	SIDE(91.5)
O - A	2	TOP
H - G	2	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
O - L	6	SIDE(640.9)
L - H	6	SIDE(1456.7)
WEBS : (0.122"x3") SPIRAL NAILS		
2x4	6	
C - M	2	SIDE(217.2)
D - K	1	
E - J	2	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMV+p	MT20	4.0	4.0	
B	TTWW-m	MT20	10.0	12.0	Edge 5.00
C	TMWW-l	MT20	5.0	6.0	2.50 2.25
D	TMWW-w	MT20	3.0	6.0	
E	TMWW-l	MT20	5.0	6.0	2.50 2.25
F	TTWW-m	MT20	10.0	12.0	Edge 5.00
G	TMV+p	MT20	4.0	4.0	
H	BMVW1-l	MT20	8.0	12.0	4.75 5.25
I	BMVW-w	MT20	3.0	8.0	
J	BMVW-t	MT20	6.0	9.0	
K	BMVW-w	MT20	7.0	12.0	4.25 6.00
L	BS-l	MT20	10.0	12.0	
M	BMVW-t	MT20	6.0	9.0	
N	BMVW-w	MT20	3.0	8.0	
O	BMVW1-l	MT20	8.0	12.0	4.75 5.25

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	GROSS REACTION	GROSS REACTION	DOWN	HORZ	UPLIFT	IN-SX	BRG	IN-SX
O	9415	0	9415	0	0	0	5-8	5-8	
H	13104	0	13104	0	0	0	5-8	5-8	

UNFACTORED REACTIONS		1ST LCASE		MAX/MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND
O	7629	4398 / 0	1628 / 0	0 / 0	1604 / 0
H	10588	6155 / 0	2225 / 0	0 / 0	2207 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.56 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x6 DRY SPF No.2 T-BRACE AT B-O, F-H

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED		FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX	MAX. UNBRAC	MEMB.	FORCE (LBS)	MAX. UNBRAC	MEMB.	FORCE (LBS)
FR-TO		FROM	TO	CS1 (LC)	FR-TO		FROM	TO	CS1 (LC)
A-B	0 / 0	-84.3	-84.3	0.08 (1)	10.00	N-B	-1311 / 0	0.05 (1)	
B-C	-19925 / 0	-84.3	-84.3	0.22 (1)	3.32	B-M	0 / 12353	0.66 (1)	
C-P	-27136 / 0	-84.3	-84.3	0.60 (1)	2.56	M-C	-4142 / 0	0.17 (1)	
P-Q	-27136 / 0	-84.3	-84.3	0.60 (1)	2.56	C-K	0 / 7988	0.43 (1)	
Q-R	-27136 / 0	-84.3	-84.3	0.60 (1)	2.56	K-D	-387 / 19	0.02 (1)	
R-D	-27136 / 0	-84.3	-84.3	0.60 (1)	2.56	K-E	0 / 6426	0.34 (1)	
D-S	-27136 / 0	-84.3	-84.3	0.60 (1)	2.56	J-E	-3521 / 0	0.15 (1)	
S-T	-27136 / 0	-84.3	-84.3	0.60 (1)	2.56	J-F	0 / 8625	0.46 (1)	
T-U	-27136 / 0	-84.3	-84.3	0.60 (1)	2.56	F-F	0 / 4007	0.21 (1)	
U-V	-27136 / 0	-84.3	-84.3	0.60 (1)	2.56	O-B	-16198 / 0	0.69 (1)	
V-E	-27136 / 0	-84.3	-84.3	0.60 (1)	2.56	F-H	-19965 / 0	0.85 (1)	
E-F	-21336 / 0	-84.3	-84.3	0.25 (1)	3.19				
F-G	0 / 0	-84.3	-84.3	0.08 (1)	10.00				
O-A	-177 / 0	0.0	0.0	0.00 (1)	7.81				
H-G	-177 / 0	0.0	0.0	0.00 (1)	7.81				

O-W	0 / 13419	-28.0	-28.0	0.40 (1)	10.00
W-X	0 / 13419	-28.0	-28.0	0.40 (1)	10.00
X-N	0 / 13419	-28.0	-28.0	0.40 (1)	10.00
N-M	0 / 13352	-28.0	-28.0	0.59 (1)	10.00
M-Y	0 / 19922	-28.0	-28.0	0.81 (1)	10.00
Y-L	0 / 19922	-28.0	-28.0	0.81 (1)	10.00
L-Z	0 / 19922	-28.0	-28.0	0.81 (1)	10.00
Z-AA	0 / 19922	-28.0	-28.0	0.81 (1)	10.00
AA-AB	0 / 19922	-28.0	-28.0	0.81 (1)	10.00
AB-K	0 / 19922	-28.0	-28.0	0.81 (1)	10.00
K-AC	0 / 21332	-28.0	-28.0	0.84 (1)	10.00
AC-AD	0 / 21332	-28.0	-28.0	0.84 (1)	10.00
AD-AE	0 / 21332	-28.0	-28.0	0.84 (1)	10.00
AE-AF	0 / 21332	-28.0	-28.0	0.84 (1)	10.00
AF-AG	0 / 21332	-28.0	-28.0	0.84 (1)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.79")
CALCULATED VERT. DEFL.(LL) = L/846 (0.34")
ALLOWABLE DEFL.(TL) = L/360 (0.79")
CALCULATED VERT. DEFL.(TL) = L/533 (0.54")

CS1: TC=0.60 (C-D:1), BC=0.84 (J-K:1), WB=0.85 (F-H:1), SSI=0.80 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

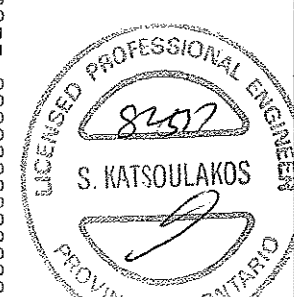
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 622 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.90 (J) (INPUT = 0.90)
JSI METAL = 0.99 (L) (INPUT = 1.00)



DWG NO. TAM 42665-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
282689	T9	1	3	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 M&T Industries, Inc. Thu Aug 24 16:30:53 2017 Page 2

ID: P_zJ0eh6kqS9gNINRH3eykdwA-DhkMDk4HxxBikwBO5O5hkw0ttWe0x3u2FABalykbS0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

WB - INDICATES BLOCKING REQUIRED

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 0.9 lbs FACTORED DOWN AT 4-2-9, AT 6-5-13, AT 8-5-13, AT 10-5-13, AT 12-5-13, AT 13-4-3, AT 15-4-3, AND AT 17-4-3, AND 0.9 lbs FACTORED DOWN AT 19-7-7 ON TOP CHORD, AND 7.7 lbs FACTORED DOWN AT 5-13, AT 2-5-13, AT 4-5-13, 3081.1 lbs FACTORED DOWN AT 6-0-0, AT 6-5-13, 1861.4 lbs FACTORED DOWN AT 7-11-4, AT 8-5-13, 1861.4 lbs FACTORED DOWN AT 9-11-4, AT 10-5-13, 1861.4 lbs FACTORED DOWN AT 11-11-4, AT 12-5-13, AT 13-4-3, 1861.4 lbs FACTORED DOWN AT 13-11-4, AT 15-4-3, 1861.4 lbs FACTORED DOWN AT 15-11-4, AT 17-4-3, 1861.4 lbs FACTORED DOWN AT 17-11-4, AT 19-4-3, 1861.4 lbs FACTORED DOWN AT 19-11-4, 1861.4 lbs FACTORED DOWN AT 21-1-0, AT 21-4-3, AND 1861.4 lbs FACTORED DOWN AT 22-2-12, AND 7.7 lbs FACTORED DOWN AT 23-4-3 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

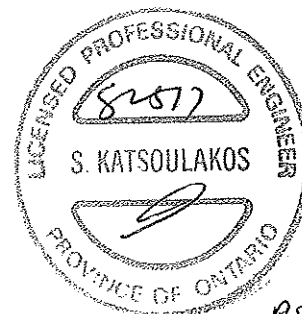
LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO			FR-TO		
AG-AH	0 / 21332	-28.0	-28.0 0.84 (1)	10.00			
AH-J	0 / 21332	-28.0	-28.0 0.84 (1)	10.00			
J-I	0 / 16745	-28.0	-28.0 0.67 (1)	10.00			
I-AI	0 / 16540	-28.0	-28.0 0.83 (1)	10.00			
AI-AJ	0 / 16540	-28.0	-28.0 0.83 (1)	10.00			
AJ-AK	0 / 16540	-28.0	-28.0 0.83 (1)	10.00			
AK-AL	0 / 16540	-28.0	-28.0 0.83 (1)	10.00			
AL-H	0 / 16540	-28.0	-28.0 0.83 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
B	4-2-9	-1	-1	---	FRONT	VERT	TOTAL
F	19-7-7	-1	-1	---	FRONT	VERT	TOTAL
I	19-4-3	---	---	---	FRONT	VERT	TOTAL
J	17-11-4	-1861	-1861	---	BACK	VERT	TOTAL
K	11-11-4	-1861	-1861	---	BACK	VERT	TOTAL
L	7-11-4	-1861	-1861	---	BACK	VERT	TOTAL
M	6-0-0	-3081	-3081	---	BACK	VERT	TOTAL
N	4-5-13	---	---	---	FRONT	VERT	TOTAL
P	6-5-13	---	---	---	FRONT	VERT	TOTAL
Q	8-5-13	---	---	---	FRONT	VERT	TOTAL
R	10-5-13	---	---	---	FRONT	VERT	TOTAL
S	12-5-13	---	---	---	FRONT	VERT	TOTAL
T	13-4-3	---	---	---	FRONT	VERT	TOTAL
U	15-4-3	---	---	---	FRONT	VERT	TOTAL
V	17-4-3	---	---	---	FRONT	VERT	TOTAL
W	5-13	-4	-8	---	FRONT	VERT	TOTAL
X	2-5-13	---	---	---	FRONT	VERT	TOTAL
Y	6-5-13	---	---	---	FRONT	VERT	TOTAL
Z	8-5-13	---	---	---	FRONT	VERT	TOTAL
AA	9-11-4	-1861	-1861	---	BACK	VERT	TOTAL
AB	10-5-13	---	---	---	FRONT	VERT	TOTAL
AC	12-5-13	---	---	---	FRONT	VERT	TOTAL
AD	13-4-3	---	---	---	FRONT	VERT	TOTAL
AE	13-11-4	-1861	-1861	---	BACK	VERT	TOTAL
AF	15-4-3	---	---	---	FRONT	VERT	TOTAL
AG	15-11-4	-1861	-1861	---	BACK	VERT	TOTAL
AH	17-4-3	---	---	---	FRONT	VERT	TOTAL
AI	19-11-4	-1861	-1861	---	BACK	VERT	TOTAL
AJ	21-1-0	-1861	-1861	---	BACK	VERT	TOTAL
AJ	21-4-3	---	---	---	FRONT	VERT	TOTAL
AK	22-2-12	-1861	-1861	---	BACK	VERT	TOTAL
AL	23-4-3	-4	-8	---	FRONT	VERT	TOTAL



DWG NO. TAM 42665-17
STRUCTURAL
COMPONENT ONLY

WB - INDICATES BLOCKING REQUIRED

HANGERS NOTES

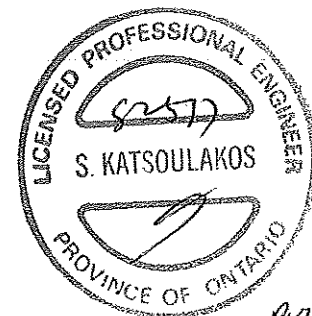
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 0.9 lbs FACTORED DOWN AT 4-2-9, AT 6-5-13, AT 8-5-13, AT 10-5-13, AT 11-11-0, AT 13-4-3, AT 15-4-3, AND AT 17-4-3, AND 0.9 lbs FACTORED DOWN AT 19-7-7 ON TOP CHORD, AND 7.7 lbs FACTORED DOWN AT 5-13, AT 2-5-13, AT 4-5-13, 3077.7 lbs FACTORED DOWN AT 6-0-0, AT 6-5-13, 1861.4 lbs FACTORED DOWN AT 7-11-4, AT 8-5-13, 1861.4 lbs FACTORED DOWN AT 9-11-4, AT 10-5-13, AT 11-11-0, 1861.4 lbs FACTORED DOWN AT 11-11-4, AT 13-4-3, 1861.4 lbs FACTORED DOWN AT 13-11-4, AT 15-4-3, 1861.4 lbs FACTORED DOWN AT 15-11-4, AT 17-4-3, 1861.4 lbs FACTORED DOWN AT 17-11-4, AT 19-4-3, 1861.4 lbs FACTORED DOWN AT 19-11-4, 1861.4 lbs FACTORED DOWN AT 21-1-0, AT 21-4-3, AND 1861.4 lbs FACTORED DOWN AT 22-2-12, AND 7.7 lbs FACTORED DOWN AT 23-4-3 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS						
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED MAX. CSI (LC)				
FR-TO		FROM TO			FR-TO						
J-I	0 / 16744	-28.0	-28.0	0.67 (1)	10.00						
I-AG	0 / 16539	-28.0	-28.0	0.83 (1)	10.00						
AG-AH	0 / 16539	-28.0	-28.0	0.83 (1)	10.00						
AH-AI	0 / 16539	-28.0	-28.0	0.83 (1)	10.00						
AI-AJ	0 / 16539	-28.0	-28.0	0.83 (1)	10.00						
AJ-H	0 / 16539	-28.0	-28.0	0.83 (1)	10.00						
FACTORED CONCENTRATED LOADS (LBS)											
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE				
B	4-2-9	-1	-1	---	FRONT	VERT	TOTAL				
D	11-11-0	---	---	---	FRONT	VERT	TOTAL				
F	19-7-7	-1	-1	---	FRONT	VERT	TOTAL				
I	19-4-3	---	---	---	FRONT	VERT	TOTAL				
J	17-11-4	-1861	-1861	---	BACK	VERT	TOTAL				
K	11-11-0	---	---	---	FRONT	VERT	TOTAL				
L	11-11-4	-1861	-1861	---	BACK	VERT	TOTAL				
M	7-11-4	-1861	-1861	---	BACK	VERT	TOTAL				
N	6-0-0	-3078	-3078	---	TOP	VERT	TOTAL				
P	4-5-13	---	---	---	FRONT	VERT	TOTAL				
Q	6-5-13	---	---	---	FRONT	VERT	TOTAL				
R	8-5-13	---	---	---	FRONT	VERT	TOTAL				
S	10-5-13	---	---	---	FRONT	VERT	TOTAL				
T	13-4-3	---	---	---	FRONT	VERT	TOTAL				
U	15-4-3	---	---	---	FRONT	VERT	TOTAL				
V	17-4-3	---	---	---	FRONT	VERT	TOTAL				
W	5-13	-4	-8	---	FRONT	VERT	TOTAL				
X	2-5-13	---	---	---	FRONT	VERT	TOTAL				
Y	6-5-13	---	---	---	FRONT	VERT	TOTAL				
Z	8-5-13	---	---	---	FRONT	VERT	TOTAL				
AA	9-11-4	-1861	-1861	---	BACK	VERT	TOTAL				
AB	10-5-13	---	---	---	FRONT	VERT	TOTAL				
AC	13-4-3	---	---	---	FRONT	VERT	TOTAL				
AD	13-11-4	-1861	-1861	---	BACK	VERT	TOTAL				
AE	15-4-3	---	---	---	FRONT	VERT	TOTAL				
AF	15-11-4	-1861	-1861	---	BACK	VERT	TOTAL				
AG	17-4-3	---	---	---	FRONT	VERT	TOTAL				
AH	19-11-4	-1861	-1861	---	BACK	VERT	TOTAL				
AI	21-1-0	-1861	-1861	---	BACK	VERT	TOTAL				
AJ	21-4-3	---	---	---	FRONT	VERT	TOTAL				
AK	22-2-12	-1861	-1861	---	BACK	VERT	TOTAL				
AL	23-4-3	-4	-8	---	FRONT	VERT	TOTAL				

DWG NO. TAM 4/2884-17
STRUCTURAL
COMPONENT ONLY



PKZ

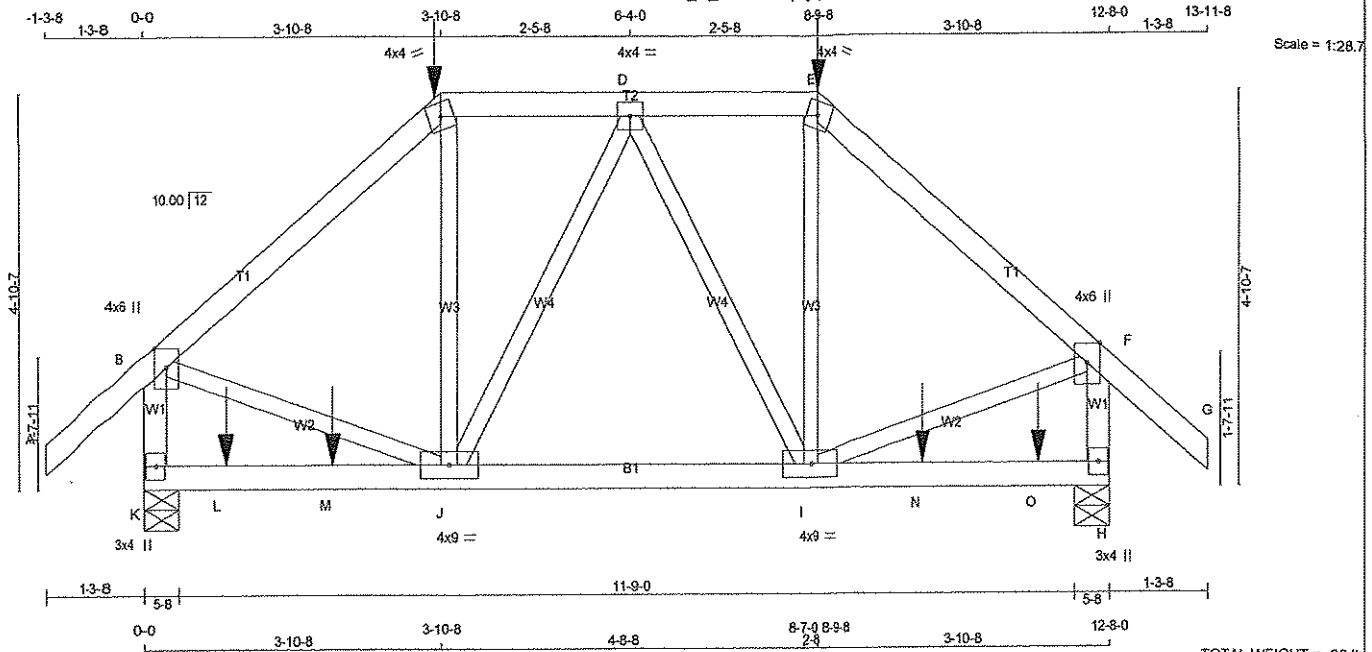
DWG NO. TAM 42884-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
282689	T10	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 6.030 S Oct 5 2016 MiTek Industries, Inc. Thu Aug 24 16:30:54 2017 Page 1

ID: P_zJoeh6kqSgNiNRH3eykdwA-UulkQ35viFJZy4mae5cwH8UGnGwfiYA1Hvv86kykbS?



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
K - B	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
K - H	2x4	DRY	No.2	SPF	

ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	
-----------------	-----	-----	------	-----	--

DRY: SEASONED LUMBER

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTW-m	MT20	4.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TTW-m	MT20	4.0	4.0		
F	TMVW+p	MT20	4.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	9.0		
J	BMWW-t	MT20	4.0	9.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 138.5 lbs FACTORED DOWN AT 3-10-8, AND 138.5 lbs FACTORED DOWN AT 8-9-8 ON TOP CHORD, AND 34.1 lbs FACTORED DOWN AT 1-0-12, 481.7 lbs FACTORED DOWN AT 2-5-8, AND 481.7 lbs FACTORED DOWN AT 10-2-8, AND 35.7 lbs FACTORED DOWN AT 11-8-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.



DWG NO. TAM 4266617
STRUCTURAL
COMPONENT ONLY

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS								
	FACTORED		MAXIMUM FACTORED			INPUT	REQD	
	GROSS REACTION		GROSS REACTION			BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
K	1467	0	1467	0	0	5-8	5-8	
H	1468	0	1468	0	0	5-8	5-8	

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
K	1165	711/0	222/0	0/0	0/0	231/0	0/0
H	1166	711/0	223/0	0/0	0/0	232/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.68 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. (PLF)	MAX. (LC)	MEMB.	FORCE (LBS)	MAX. (LC)	MEMB.
FR-TO	0/37	-84.3	-84.3 0.13 (1)	J-C	0/321	0.08 (2)	J-C
A-B	-1094/0	-84.3	-84.3 0.27 (1)	I-E	0/321	0.08 (2)	I-E
B-C	-845/0	-84.3	-84.3 0.08 (1)	B-J	0/882	0.22 (1)	B-J
C-D	-845/0	-84.3	-84.3 0.08 (1)	I-F	0/882	0.22 (1)	I-F
D-E	-1094/0	-84.3	-84.3 0.27 (1)	J-D	-130/0	0.05 (1)	J-D
E-F	-1094/0	-84.3	-84.3 0.27 (1)	D-I	-130/0	0.05 (1)	D-I
F-G	0/37	-84.3	-84.3 0.13 (1)				
K-B	-1258/0	0.0	0.0 0.15 (1)				
H-F	-1258/0	0.0	0.0 0.15 (1)				
K-L	0/0	-28.0	-28.0 0.60 (1)				
L-M	0/0	-28.0	-28.0 0.60 (1)				
M-J	0/0	-28.0	-28.0 0.60 (1)				
J-I	0/805	-28.0	-28.0 0.43 (1)				
I-N	0/0	-28.0	-28.0 0.60 (1)				
N-O	0/0	-28.0	-28.0 0.60 (1)				
O-H	0/0	-28.0	-28.0 0.60 (1)				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE
C	3-10-8	-13	-14	-	FRONT	VERT	DEAD
C	3-10-8	-126	-126	-	FRONT	VERT	SNOW
E	8-9-8	-13	-14	-	FRONT	VERT	DEAD
E	8-9-8	-126	-126	-	FRONT	VERT	SNOW
L	1-0-12	-20	-34	-	BACK	VERT	TOTAL
M	2-5-8	-482	-482	-	BACK	VERT	TOTAL
N	10-2-8	-482	-482	-	BACK	VERT	TOTAL
O	11-8-12	-20	-36	-	BACK	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
DL =	3.0	PSF	
BOT CH.	LL =	10.5	PSF
DL =	7.0	PSF	
TOTAL LOAD =	46.1	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***

ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.42")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.42")
CALCULATED VERT. DEFL.(TL) = L/999 (0.12")

CSI: TC=0.27 (B-C:1), BC=0.60 (J-K:1), WB=0.22 (B-J:1), SSI=0.35 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MT20	618	354	1687 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

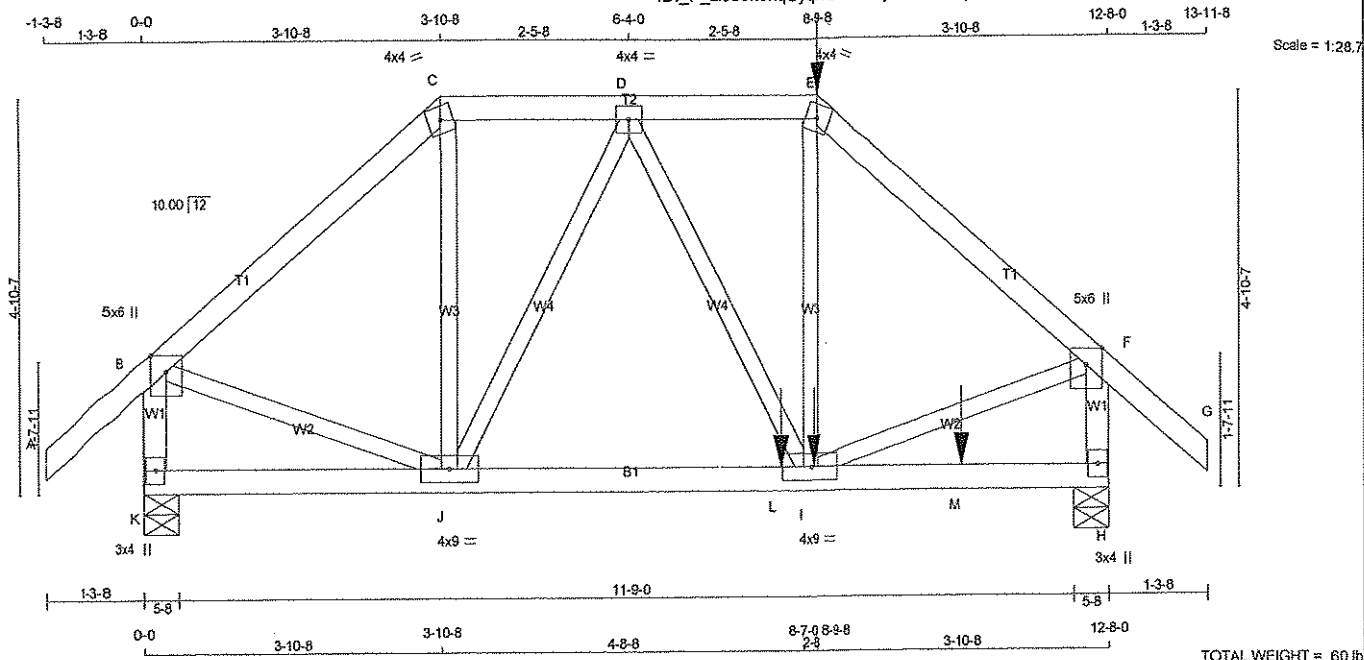
JSI GRIP= 0.85 (B) (INPUT = 0.90)
JSI METAL= 0.27 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
282690	T10Z	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 Mitek Industries, Inc. Thu Aug 24 16:34:10 2017 Page 1

ID: P_zJoeh6kqSyqNNRH3eykdwA-lb8qZ5sYT7ITC1F7F7uDTbm7HsJgAzuLPOWzNykboX



TOTAL WEIGHT = 60 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
K - B	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
K - H	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTW-m	MT20	4.0	4.0		
D	TMVW-t	MT20	4.0	4.0		
E	TTW-m	MT20	4.0	4.0		
F	TMVW+p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMVW-t	MT20	4.0	9.0		
J	BMVW-t	MT20	4.0	9.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 225.2 lbs FACTORED DOWN AT 8-9-8 ON TOP CHORD, AND 725.4 lbs FACTORED DOWN AT 8-3-8, AND 34.1 lbs FACTORED DOWN AT 8-8-12, AND 42.0 lbs FACTORED DOWN AT 10-8-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
K	1157	0	1157	0	0	5-8	5-8	5-8	5-8
H	1493	0	1493	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX / MIN. COMPONENT REACTIONS		1ST LCASE		MAX / MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
K	914	565 / 0	169 / 0	0 / 0	0 / 0	180 / 0	0 / 0
H	1175	733 / 0	213 / 0	0 / 0	0 / 0	230 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.26 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX. FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1	MAX	MAX.	MEMB.	FORCE (LBS)	MAX	MAX.
FR-TO		FROM	TO	LENGTH	FR-TO				
A-B	0 / 37	-84.3	-84.3	0.13 (1)	10.00	J-C	0 / 353	0.09 (1)	
B-C	-944 / 0	-84.3	-84.3	0.26 (1)	6.01	I-E	0 / 372	0.09 (1)	
C-D	-731 / 0	-84.3	-84.3	0.08 (1)	6.25	B-J	0 / 761	0.19 (1)	
D-E	-1026 / 0	-84.3	-84.3	0.09 (1)	6.07	I-F	0 / 1071	0.27 (1)	
E-F	-1329 / 0	-84.3	-84.3	0.28 (1)	5.26	J-D	-452 / 0	0.21 (1)	
F-G	0 / 37	-84.3	-84.3	0.13 (1)	10.00	D-I	0 / 193	0.05 (1)	
K-B	-1124 / 0	0.0	0.0	0.13 (1)	7.45				
H-F	-1468 / 0	0.0	0.0	0.17 (1)	6.71				
K-J	0 / 0	-28.0	-28.0	0.17 (2)	10.00				
J-L	0 / 938	-28.0	-28.0	0.42 (1)	10.00				
L-I	0 / 938	-28.0	-28.0	0.42 (1)	10.00				
I-M	0 / 0	-28.0	-28.0	0.26 (2)	10.00				
M-H	0 / 0	-28.0	-28.0	0.26 (2)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE
E	8-9-8	-225	-225	---	BACK	VERT	TOTAL
I	8-8-12	-20	-34	---	BACK	VERT	TOTAL
L	8-3-8	-725	-725	---	BACK	VERT	TOTAL
M	10-8-12	-24	-42	---	BACK	VERT	TOTAL



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.42")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.05")
ALLOWABLE DEFL. (TL) = $L/360$ (0.42")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.07")

CSI: TC=0.28 (E-F:1), BC=0.42 (I-J:1), WB=0.27 (F-I:1), SS=0.64 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	618	354	1667

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.70 (B) (INPUT = 0.90)

JSI METAL= 0.25 (F) (INPUT = 1.00)

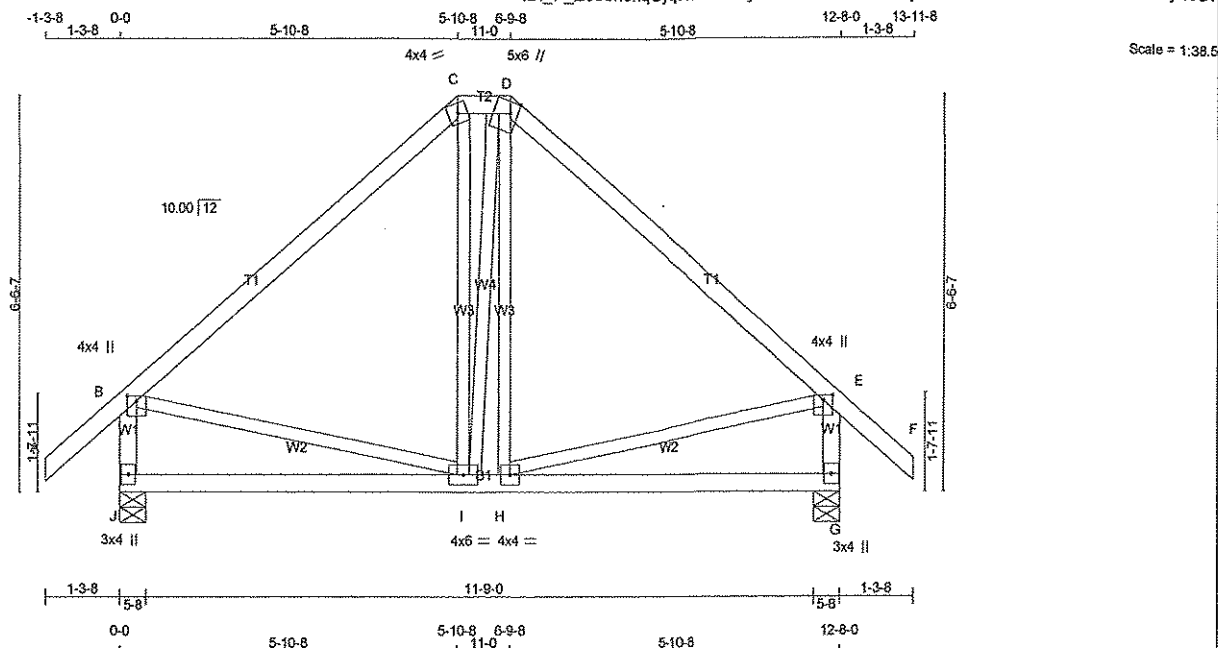
DWG NO. TAM 42871-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
282689	T11	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 Mitek Industries, Inc. Thu Aug 24 16:30:54 2017 Page 1

ID: P_zlJoeh6kqSyqNINRH3eykdwA-UulkQ35viFJZy4mae5cwH8UF5G?7laE1Hvv86kykbs?



TOTAL WEIGHT = 64 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
J - B	2x4	DRY	No.2	SPF	
G - E	2x4	DRY	No.2	SPF	
J - G	2x4	DRY	No.2	SPF	

ALL WEBS	2x3	DRY	No.2	SPF	
----------	-----	-----	------	-----	--

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00	
C	TTW-m	MT20	4.0	4.0			
D	TTWW+m	MT20	5.0	6.0	2.25	1.50	
E	TMVW+p	MT20	4.0	4.0	1.00	2.00	
G	BMV1+p	MT20	3.0	4.0			
H	BMWW-t	MT20	4.0	4.0			
I	BMWW-t	MT20	4.0	6.0			
J	BMV1+p	MT20	3.0	4.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
J	828	0	828	0	5-8	5-8
G	828	0	828	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	663	395 / 0	133 / 0	0 / 0	0 / 0	135 / 0	0 / 0
G	663	395 / 0	133 / 0	0 / 0	0 / 0	135 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO 2 OR BETTER AT JOINT(S) J, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	LC1	MAX. UNBRAC	
FR-TO		FROM	TO	LENGTH	FR-TO				
A-B	0 / 37	-84.3	-84.3	0.12 (1)	10.00	I-C	0 / 149	0.03 (3)	
B-C	-491 / 0	-84.3	-84.3	0.38 (1)	6.25	I-D	0 / 30	0.01 (3)	
C-D	-378 / 0	-84.3	-84.3	0.01 (1)	6.25	H-D	0 / 119	0.03 (3)	
D-E	-491 / 0	-84.3	-84.3	0.38 (1)	6.25	B-I	0 / 395	0.09 (1)	
E-F	0 / 37	-84.3	-84.3	0.12 (1)	10.00	H-E	0 / 395	0.09 (1)	
J-B	-760 / 0	0.0	0.0	0.08 (1)	7.81				
G-E	-761 / 0	0.0	0.0	0.08 (1)	7.81				
J-I	0 / 0	-28.0	-28.0	0.27 (3)	10.00				
I-H	0 / 377	-28.0	-28.0	0.32 (2)	10.00				
H-G	0 / 0	-28.0	-28.0	0.25 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.42")
CALCULATED VERT. DEFL.(LL)= 1/999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.42")
CALCULATED VERT. DEFL.(TL)= 1/999 (0.08")

CSI: TC=0.38 (B-C-1), BC=0.32 (H-I-2), WB=0.09 (B-I-1), SSI=0.15 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.70 (B) (INPUT = 0.90)
JSI METAL= 0.19 (B) (INPUT = 1.00)



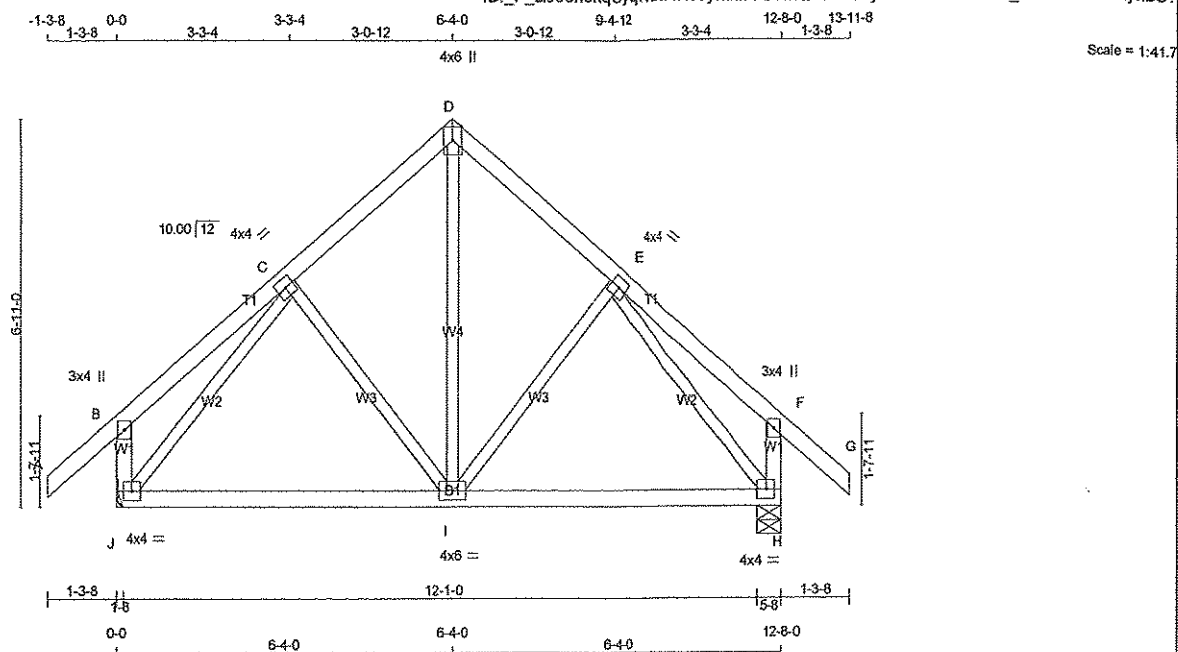
DWG NO. TAM 4286217
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
282689	T12	3	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Thu Aug 24 16:30:54 2017 Page 1

ID: _P_zJoeh6kqSyqHNRH3eykdwA-UulkQ35viFJZy4mae5cwH8UlxG_DIXC1Hvv86kykbs?



TOTAL WEIGHT = 3 X 61 = 182 lb [MJJF]

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
J	828	0	828	0	0	0
H	828	0	828	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	663	395 / 0	133 / 0	0 / 0	0 / 0	135 / 0	0 / 0
H	663	395 / 0	133 / 0	0 / 0	0 / 0	135 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)		
FR-TO		FROM TO			FR-TO				
A-B	0 / 37	-84.3	-84.3	0.12 (1)	10.00	I-D	0 / 393	0.09 (1)	
B-C	0 / 19	-84.3	-84.3	0.13 (1)	10.00	I-E	-112 / 39	0.05 (1)	
C-D	-490 / 0	-84.3	-84.3	0.10 (1)	6.25	C-I	-112 / 39	0.05 (1)	
D-E	-490 / 0	-84.3	-84.3	0.10 (1)	6.25	J-C	-693 / 0	0.28 (1)	
E-F	0 / 19	-84.3	-84.3	0.13 (1)	10.00	E-H	-693 / 0	0.28 (1)	
F-G	0 / 37	-84.3	-84.3	0.12 (1)	10.00				
J-B	-219 / 0	0.0	0.0	0.02 (1)	7.81				
H-F	-219 / 0	0.0	0.0	0.02 (1)	7.81				
J-I	0 / 433	-28.0	-28.0	0.37 (2)	10.00				
I-H	0 / 433	-28.0	-28.0	0.37 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.42")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.42")
CALCULATED VERT. DEFL.(TL) = L/999 (0.08")

CSI: TC=0.13 (E-F:1), BC=0.37 (I-J:2), WB=0.28 (E-H:1), SS=0.15 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PL)
MT20	618	354
	1687	822
	2284	1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.80 (C) (INPUT = 0.90)
JSI METAL = 0.25 (E) (INPUT = 1.00)

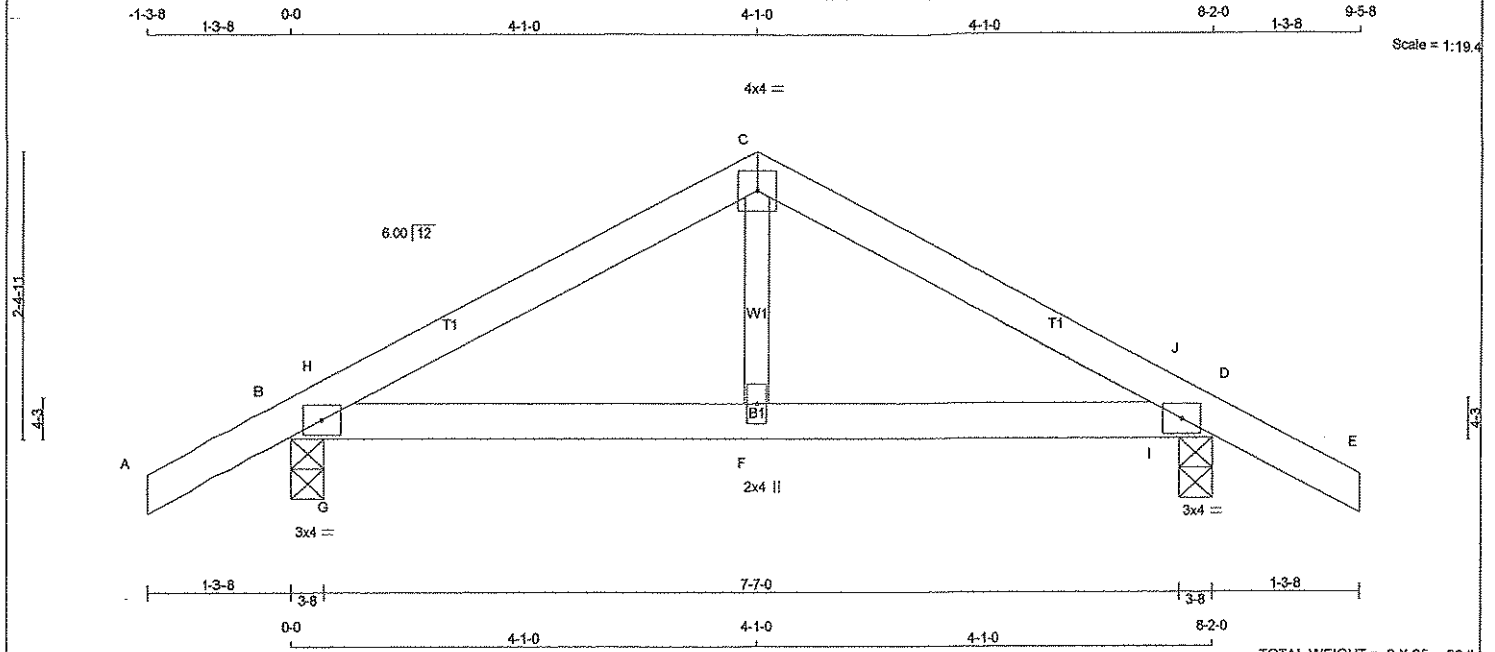


DWG NO. TAM 4266817
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	43954	DRWG NO.
282689	T13	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MTek Industries, Inc. Thu Aug 24 16:30:54 2017 Page 1
ID: P_zlJoeh6kgSyqNUNRH3eykdwA-UulkQ35vIFJzy4mae5cwH8UIVG0llbi1Hvv86kykbs?



TOTAL WEIGHT = 2 X 25 = 50 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
B - D	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0			
C	TTW-p	MT20	4.0	4.0			
D	TMB1-I	MT20	3.0	4.0			
F	BMW+w	MT20	2.0	4.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS							
	FACTORED		MAXIMUM FACTORED		INPUT	REQD	
	GROSS REACTION		GROSS REACTION		BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
B	573	0	573	0	0	3-8	3-8
D	573	0	573	0	0	3-8	3-8

UNFACTORED REACTIONS

1ST LCASE	MAX / MIN. COMPONENT REACTIONS	JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	454	278 / 0	86 / 0	0 / 0	0 / 0	0 / 0	90 / 0	0 / 0	0 / 0
D	454	278 / 0	86 / 0	0 / 0	0 / 0	0 / 0	90 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO			FR-TO			
A-B	0 / 24	-84.3	-84.3	0.11 (1)	10.00	F-C	0 / 250	0.08 (2)
B-H	-528 / 0	-84.3	-84.3	0.04 (1)	6.25	G-H	-234 / 76	0.09 (1)
H-C	-559 / 0	-84.3	-84.3	0.16 (1)	6.25	I-J	-234 / 76	0.09 (1)
C-J	-559 / 0	-84.3	-84.3	0.16 (1)	6.25			
J-D	-528 / 0	-84.3	-84.3	0.04 (1)	6.25			
D-E	0 / 24	-84.3	-84.3	0.11 (1)	10.00			
B-G	0 / 495	-28.0	-28.0	0.21 (1)	10.00			
G-F	0 / 495	-28.0	-28.0	0.21 (1)	10.00			
F-I	0 / 495	-28.0	-28.0	0.21 (1)	10.00			
I-D	0 / 495	-28.0	-28.0	0.21 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
DL	=	3.0	PSF	
BOT CH.	LL	=	10.5	PSF
DL	=	7.0	PSF	
TOTAL LOAD	=	46.1	PSF	

SPACING = 24.0 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.27")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.27")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.16 (C-J:1), BC=0.21 (F-I:1), WB=0.05 (C-F:2), SS=0.19 (D-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP(DRY)		SHEAR		SECTION
	(PSI)		(PLI)		(PLI)
	MAX	MIN	MAX	MIN	MAX MIN
MT20	618	354	1667	822	2284 1656

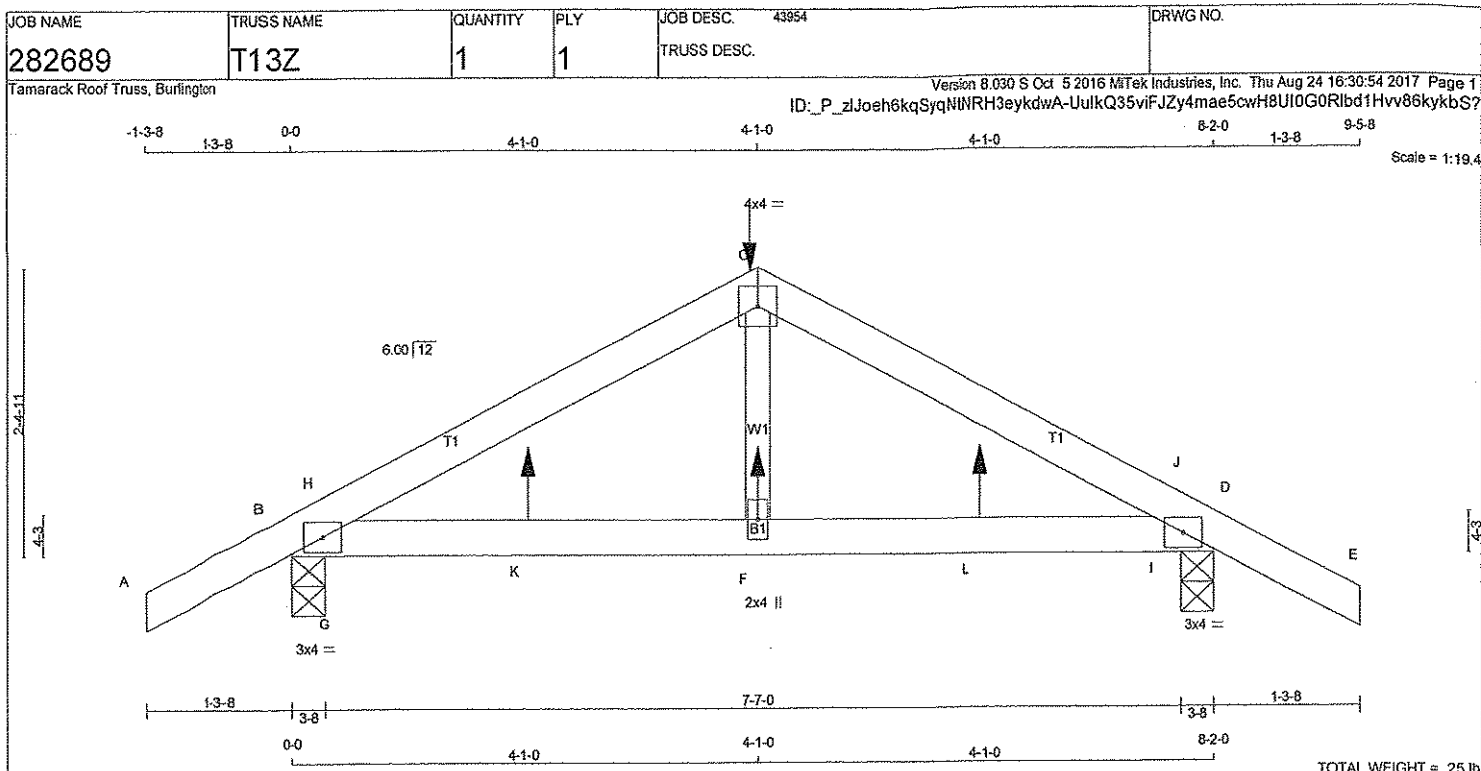
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.52 (B) (INPUT = 0.90)
JSI METAL= 0.20 (B) (INPUT = 1.00)



DRWG NO. TAM 42869-17
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 25 lb (M)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
B - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
SPF			
DRY: SEASONED LUMBER.			

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW-p	MT20	4.0	4.0		
D	TMB1-I	MT20	3.0	4.0		
F	BMW+w	MT20	2.0	4.0		

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 2.4 lbs FACTORED DOWN AND 99.2 lbs FACTORED UP AT 4-1-0 ON TOP CHORD, AND 16.6 lbs FACTORED UP AT 2-1-0, AND 16.6 lbs FACTORED UP AT 4-1-0, AND 16.6 lbs FACTORED UP AT 6-1-0 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
B	560	0	560	0	3-8	3-8
D	560	0	560	0	3-8	3-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	446	269 / 0	87 / 0	0 / 0	0 / 0	90 / 0	0 / 0
D	446	269 / 0	87 / 0	0 / 0	0 / 0	90 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX. (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. (LC)
FR-TO					FR-TO		
A-B	0 / 24	-84.3	-84.3 0.12 (1)	10.00	F-C	0 / 243	0.08 (7)
B-H	-501 / 0	-84.3	-84.3 0.07 (5)	6.25	G-H	-255 / 76	0.00 (1)
H-C	-537 / 0	-84.3	-84.3 0.19 (1)	6.25	I-J	-255 / 76	0.00 (1)
C-J	-537 / 0	-84.3	-84.3 0.19 (1)	6.25			
J-D	-501 / 0	-84.3	-84.3 0.07 (5)	6.25			
D-E	0 / 24	-84.3	-84.3 0.12 (1)	10.00			
B-G	0 / 476	-28.0	-28.0 0.23 (1)	10.00			
G-K	0 / 476	-28.0	-28.0 0.23 (1)	10.00			
K-F	0 / 476	-28.0	-28.0 0.23 (1)	10.00			
F-L	0 / 476	-28.0	-28.0 0.23 (1)	10.00			
L-I	0 / 476	-28.0	-28.0 0.23 (1)	10.00			
I-D	0 / 476	-28.0	-28.0 0.23 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
C	4-1-0	-1	-2	99	BACK	VERT	TOTAL
F	4-1-0	9	-	17	BACK	VERT	TOTAL
K	2-1-0	9	-	17	BACK	VERT	TOTAL
L	6-1-0	9	-	17	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.27")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.27")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.19 (C-J:1), BC=0.23 (F-I:1), WB=0.06 (C-F:7), SSI=0.22 (D-I:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

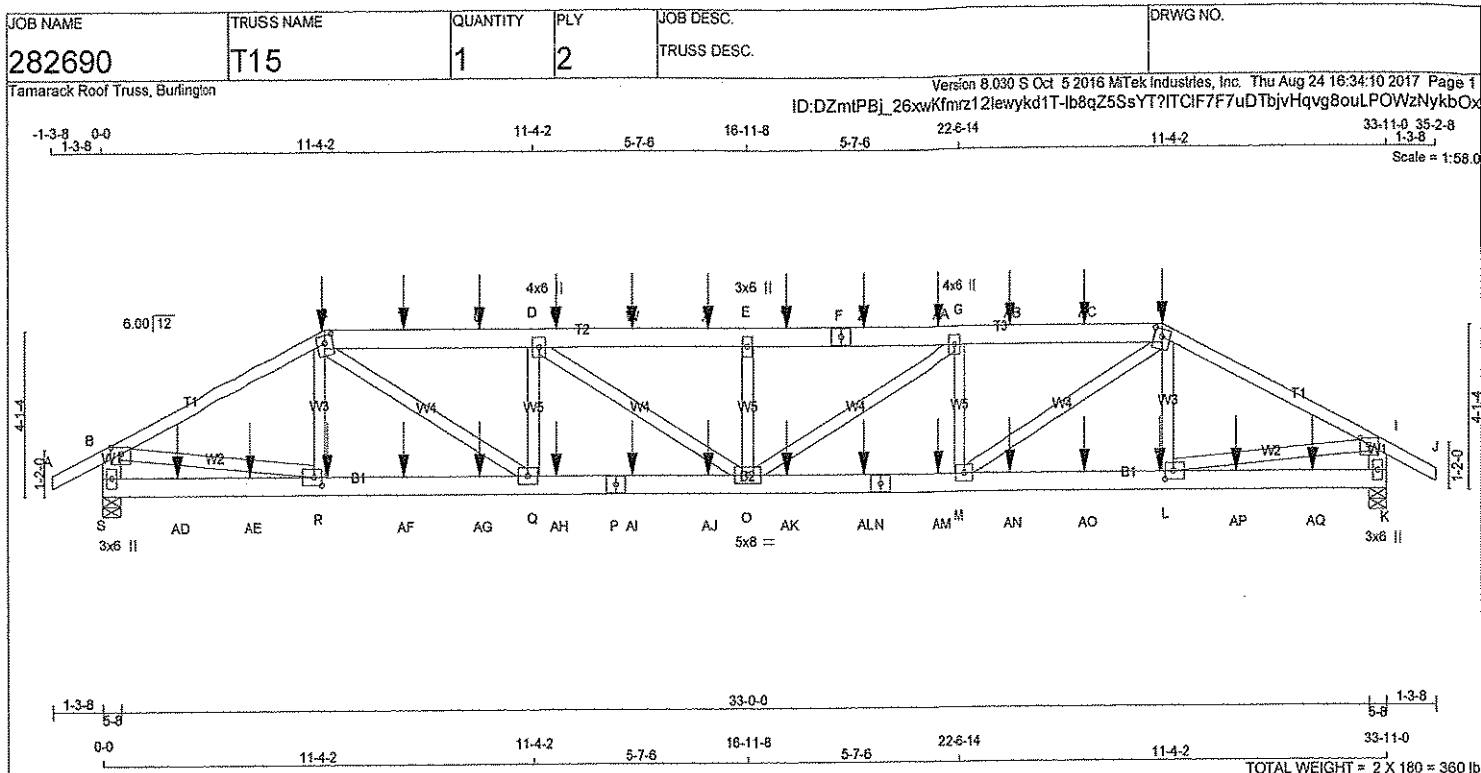
PLATE	GRIP(DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)	(PL)
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg

JSI GRIP= 0.51 (B) (INPUT = 0.90)
JSI METAL = 0.19 (B) (INPUT = 1.00)





LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - F	2x6	DRY	No.2
F - H	2x6	DRY	No.2
H - J	2x4	DRY	No.2
S - B	2x6	DRY	No.2
K - I	2x6	DRY	No.2
S - P	2x6	DRY	No.2
P - N	2x6	DRY	No.2
N - K	2x6	DRY	No.2
ALL WEBS	2x4	DRY	No.2
EXCEPT			SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	12	SIDE(61.0)
H - J	12	SIDE(61.0)
C - F	2	SIDE(61.0)
F - H	2	SIDE(61.0)
S - B	2	TOP
K - I	2	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
S - P	2	SIDE(183.1)
P - N	2	SIDE(0.0)
N - K	2	SIDE(183.1)

WEBS : (0.122"x3") SPIRAL NAILS		
R - C	1	SIDE(1.2)
L - H	1	SIDE(1.2)
2x4	1	6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	2.00	3.00
C	TTWW+m	MT20	5.0	6.0	2.25	2.50
D	TMVW+t	MT20	4.0	6.0		
E	TMVW+w	MT20	3.0	6.0		
F	TS-I	MT20	5.0	6.0		
G	TMVW+t	MT20	4.0	6.0		
H	TTWW+m	MT20	5.0	6.0	2.25	2.50
I	TMVW-p	MT20	5.0	6.0	2.00	3.00
K	BMVW-l	MT20	3.0	6.0		
L	BMVW-l	MT20	5.0	6.0	2.50	2.50
M	BMVW-l	MT20	5.0	6.0		
N	BS-I	MT20	5.0	6.0		
O	BMVW-w	MT20	5.0	8.0		
P	BS-I	MT20	5.0	6.0		
Q	BMVW-l	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
S	3334	0	3334	0	5-8	5-8
K	3334	0	3334	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. LIVE		PERM. LIVE	WIND	DEAD	SOIL
	SNOW	SNOW	SNOW	SNOW				
S	2726	1530 / 0	608 / 0	0 / 0	0 / 0	0 / 0	588 / 0	0 / 0
K	2726	1530 / 0	608 / 0	0 / 0	0 / 0	0 / 0	588 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.91 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	FORCE (LBS)	MEMB.	FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 26	R-C	-314 / 241
B-C	-5081 / 0	L-H	-314 / 241
C-T	-6957 / 0	B-R	0 / 4579
T-U	-6957 / 0	L-I	0 / 4579
U-D	-6957 / 0	M-H	0 / 2901
D-V	-7712 / 0	C-Q	0 / 2901
V-W	-7712 / 0	M-G	-1353 / 0
W-X	-7712 / 0	Q-D	-1353 / 0
X-E	-7712 / 0	O-G	0 / 901
E-Y	-7712 / 0	D-O	0 / 901
Y-F	-7712 / 0	O-E	-721 / 0
F-Z	-7712 / 0		
Z-AA	-7712 / 0		
AA-G	-7712 / 0		
G-AB	-6957 / 0		
AB-AC	-6957 / 0		
AC-H	-6957 / 0		
H-I	-5081 / 0		
I-J	0 / 26		
S-B	-3218 / 0		
K-I	-3218 / 0		
S-AD	0 / 0		
AD-AE	0 / 0		
AE-R	0 / 0		
R-AF	0 / 4550		
AF-AG	0 / 4550		
AG-Q	0 / 4550		
Q-AH	0 / 6957		
AH-P	0 / 6957		
P-AI	0 / 6957		
AI-AJ	0 / 6957		
AJ-O	0 / 6957		
O-AK	0 / 6957		
AK-AL	0 / 6957		
AL-N	0 / 6957		
N-AM	0 / 6957		
AM-M	0 / 6957		
M-AN	0 / 4550		
AN-AO	0 / 4550		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
DL = 3.0 PSF	
BOT CH.	LL = 10.5 PSF
DL = 7.0 PSF	
TOTAL LOAD	= 46.1 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.13")
CALCULATED VERT. DEFL.(LL) = L/999 (0.21")
ALLOWABLE DEFL.(TL) = L/360 (1.13")
CALCULATED VERT. DEFL.(TL) = L/999 (0.34")

CSI: TC=0.49 (B-C:1), BC=0.51 (M-O:1),
WB=0.40 (B-R:1), SSI=0.14 (O-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1697 622 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (C) (INPUT = 0.90)
JSI METAL= 0.58 (N) (INPUT = 1.00)



DWG NO. TAM4287217
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
282690	T15	1	2	TRUSS DESC.	
Tamarack Roof Truss, Burlington					
Version 8.030 S Oct 5 2016 MITek Industries, Inc. Thu Aug 24 16:34:10 2017 Page 2					
ID:DZmtPBJ 26xwKfmrz12lewykd1T-lb8qZ5SsYT?ITCIf7F7uDTbjwHqvg8ouLPOWzNykboX					

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
R	BMWW-I	MT20	5.0	6.0	2.50	2.50
S	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 472.3 lbs FACTORED DOWN AT 5-10-8, 101.4 lbs FACTORED DOWN AT 7-11-4, 101.4 lbs FACTORED DOWN AT 9-11-4, 101.4 lbs FACTORED DOWN AT 11-11-4, 101.4 lbs FACTORED DOWN AT 13-11-4, 101.4 lbs FACTORED DOWN AT 15-11-4, 101.4 lbs FACTORED DOWN AT 17-11-12, 101.4 lbs FACTORED DOWN AT 19-11-12, 101.4 lbs FACTORED DOWN AT 21-11-12, 101.4 lbs FACTORED DOWN AT 23-11-12, AND 101.4 lbs FACTORED DOWN AT 25-11-12, AND 472.3 lbs FACTORED DOWN AT 28-0-8 ON TOP CHORD, AND 84.6 lbs FACTORED DOWN AT 1-11-4, 84.6 lbs FACTORED DOWN AT 3-11-4, 69.9 lbs FACTORED DOWN AT 5-11-4, 69.9 lbs FACTORED DOWN AT 7-11-4, 69.9 lbs FACTORED DOWN AT 9-11-4, 69.9 lbs FACTORED DOWN AT 11-11-4, 69.9 lbs FACTORED DOWN AT 13-11-4, 69.9 lbs FACTORED DOWN AT 15-11-4, 69.9 lbs FACTORED DOWN AT 17-11-12, 69.9 lbs FACTORED DOWN AT 19-11-12, 69.9 lbs FACTORED DOWN AT 21-11-12, 69.9 lbs FACTORED DOWN AT 23-11-12, 69.9 lbs FACTORED DOWN AT 25-11-12, 69.9 lbs FACTORED DOWN AT 27-11-12, AND 84.6 lbs FACTORED DOWN AT 29-11-12, AND 84.6 lbs FACTORED DOWN AT 31-11-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LOADING

TOTAL LOAD CASES: (4)

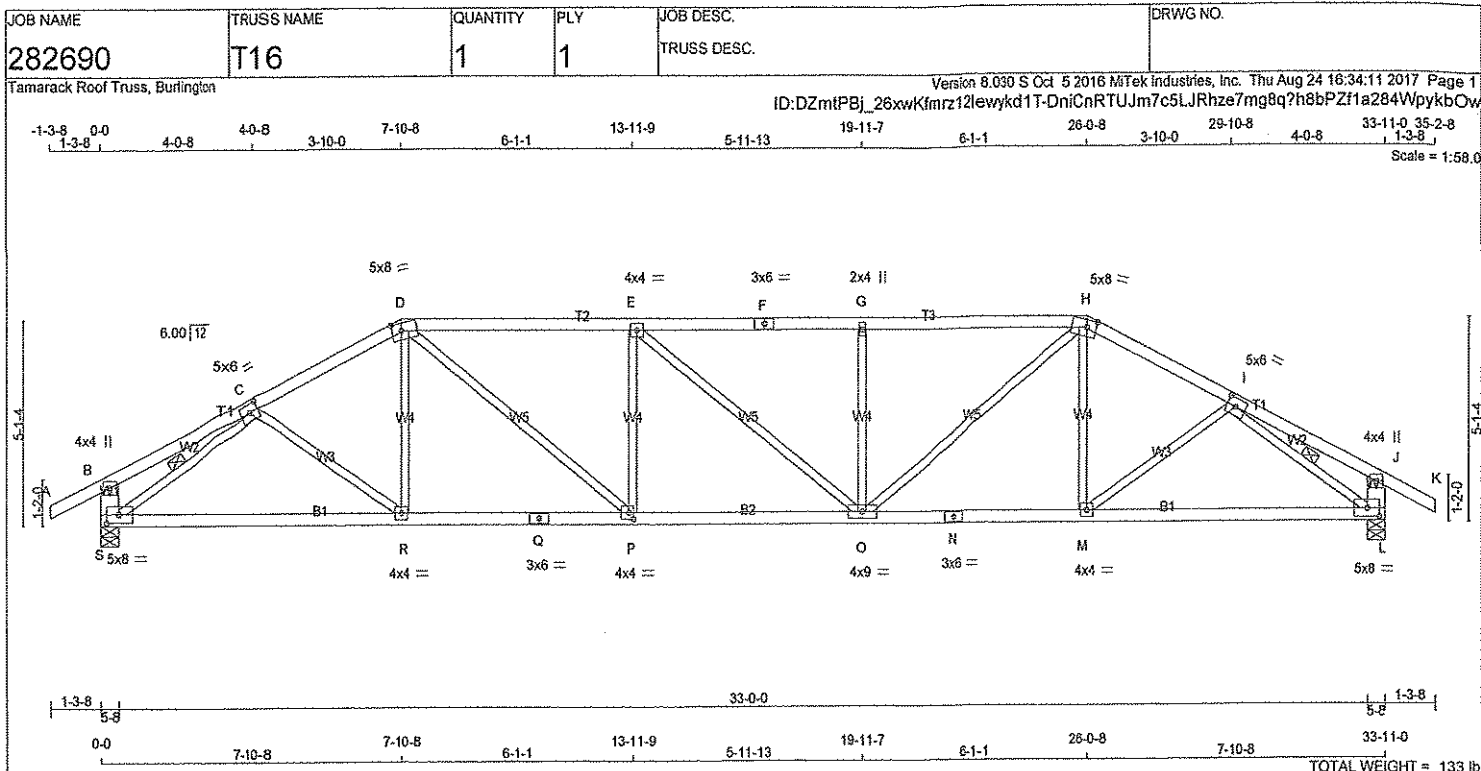
CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRAC LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO		FROM	TO		FR-TO		
AO-L	0 / 4550	-28.0	-28.0	0.34 (1)	10.00		
L-AP	0 / 0	-28.0	-28.0	0.10 (2)	10.00		
AP-AQ	0 / 0	-28.0	-28.0	0.10 (2)	10.00		
AQ-K	0 / 0	-28.0	-28.0	0.10 (2)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	5-10-8	-472	-472	---	FRONT	VERT	TOTAL
H	28-0-8	-472	-472	---	FRONT	VERT	TOTAL
L	27-11-12	-40	-70	---	FRONT	VERT	TOTAL
R	5-11-4	-40	-70	---	FRONT	VERT	TOTAL
T	7-11-4	-101	-101	---	FRONT	VERT	TOTAL
U	9-11-4	-101	-101	---	FRONT	VERT	TOTAL
V	11-11-4	-101	-101	---	FRONT	VERT	TOTAL
W	13-11-4	-101	-101	---	FRONT	VERT	TOTAL
X	15-11-4	-101	-101	---	FRONT	VERT	TOTAL
Y	17-11-12	-101	-101	---	FRONT	VERT	TOTAL
Z	19-11-12	-101	-101	---	FRONT	VERT	TOTAL
AA	21-11-12	-101	-101	---	FRONT	VERT	TOTAL
AB	23-11-12	-101	-101	---	FRONT	VERT	TOTAL
AC	25-11-12	-101	-101	---	FRONT	VERT	TOTAL
AD	1-11-4	-48	-85	---	FRONT	VERT	TOTAL
AE	3-11-4	-48	-85	---	FRONT	VERT	TOTAL
AF	7-11-4	-40	-70	---	FRONT	VERT	TOTAL
AG	9-11-4	-40	-70	---	FRONT	VERT	TOTAL
AH	11-11-4	-40	-70	---	FRONT	VERT	TOTAL
AI	13-11-4	-40	-70	---	FRONT	VERT	TOTAL
AJ	15-11-4	-40	-70	---	FRONT	VERT	TOTAL
AK	17-11-12	-40	-70	---	FRONT	VERT	TOTAL
AL	19-11-12	-40	-70	---	FRONT	VERT	TOTAL
AM	21-11-12	-40	-70	---	FRONT	VERT	TOTAL
AN	23-11-12	-40	-70	---	FRONT	VERT	TOTAL
AO	25-11-12	-40	-70	---	FRONT	VERT	TOTAL
AP	29-11-12	-48	-85	---	FRONT	VERT	TOTAL
AQ	31-11-12	-48	-85	---	FRONT	VERT	TOTAL



DWG NO. TAM 42872-17
STRUCTURAL
COMPONENT ONLY



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
S - B	2x6	DRY	No.2
L - J	2x6	DRY	No.2
S - Q	2x4	DRY	No.2
Q - N	2x4	DRY	No.2
N - L	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.75
D	TTWW-m	MT20	5.0	8.0	2.25	3.00
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	8.0	2.25	3.00
I	TMWW-t	MT20	5.0	6.0	2.50	2.75
J	TMV+p	MT20	4.0	4.0		
L	BMVW1-t	MT20	5.0	8.0	2.50	3.75
M	BMWW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	9.0		
P	BMWW-t	MT20	4.0	4.0	2.00	1.50
Q	BS-t	MT20	3.0	6.0		
R	BMWW-t	MT20	4.0	4.0		
S	BMVW1-t	MT20	5.0	8.0	2.50	3.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

BEARINGS

JT	FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
	GROSS	REACTION	GROSS	REACTION	BRG		BRG	
S	2018	0	2018	0	0	5-8	5-8	
L	2018	0	2018	0	0	5-8	5-8	

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED		SNOW	LIVE			
S	1641	937 / 0	356 / 0	0 / 0	0 / 0	347 / 0	0 / 0
L	1641	937 / 0	356 / 0	0 / 0	0 / 0	347 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-S, I-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC) UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 26	-84.3 -84.3	0.11 (1)	C-R	0 / 210	0.05 (3)	
B-C	0 / 14	-84.3 -84.3	0.17 (1)	R-D	0 / 288	0.06 (2)	
C-D	-2730 / 0	-84.3 -84.3	0.28 (1)	D-P	0 / 1135	0.26 (1)	
D-E	-3322 / 0	-84.3 -84.3	0.72 (1)	P-E	-551 / 0	0.21 (1)	
E-F	-3320 / 0	-84.3 -84.3	0.71 (1)	E-O	-2 / 0	0.00 (1)	
F-G	-3320 / 0	-84.3 -84.3	0.71 (1)	O-G	-551 / 0	0.21 (1)	
G-H	-3320 / 0	-84.3 -84.3	0.72 (1)	O-H	0 / 1134	0.26 (1)	
H-I	-2730 / 0	-84.3 -84.3	0.28 (1)	H-H	0 / 288	0.06 (2)	
I-J	0 / 14	-84.3 -84.3	0.17 (1)	M-I	0 / 210	0.05 (3)	
J-K	0 / 26	-84.3 -84.3	0.11 (1)	S-C	-2876 / 0	0.56 (1)	
S-B	-249 / 0	0.0 0.0	0.02 (1)	I-L	-2876 / 0	0.56 (1)	
L-J	-249 / 0	0.0 0.0	0.02 (1)				
S-R	0 / 2337	-28.0 -28.0	0.65 (2)				
R-Q	0 / 2430	-28.0 -28.0	0.67 (2)				
Q-P	0 / 2430	-28.0 -28.0	0.67 (2)				
P-O	0 / 3322	-28.0 -28.0	0.63 (1)				
O-N	0 / 2430	-28.0 -28.0	0.68 (2)				
N-M	0 / 2430	-28.0 -28.0	0.68 (2)				
M-L	0 / 2337	-28.0 -28.0	0.65 (2)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
DL = 3.0 PSF	
BOT CH.	LL = 10.5 PSF
DL = 7.0 PSF	
TOTAL LOAD	= 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.13")
CALCULATED VERT. DEFL.(LL) = L/999 (0.24")
ALLOWABLE DEFL.(TL)= L/360 (1.13")
CALCULATED VERT. DEFL.(TL) = L/999 (0.40")

CSI: TC=0.72 (D-E:1), BC=0.68 (M-O:2),
WB=0.56 (I-L:1), SSI=0.24 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

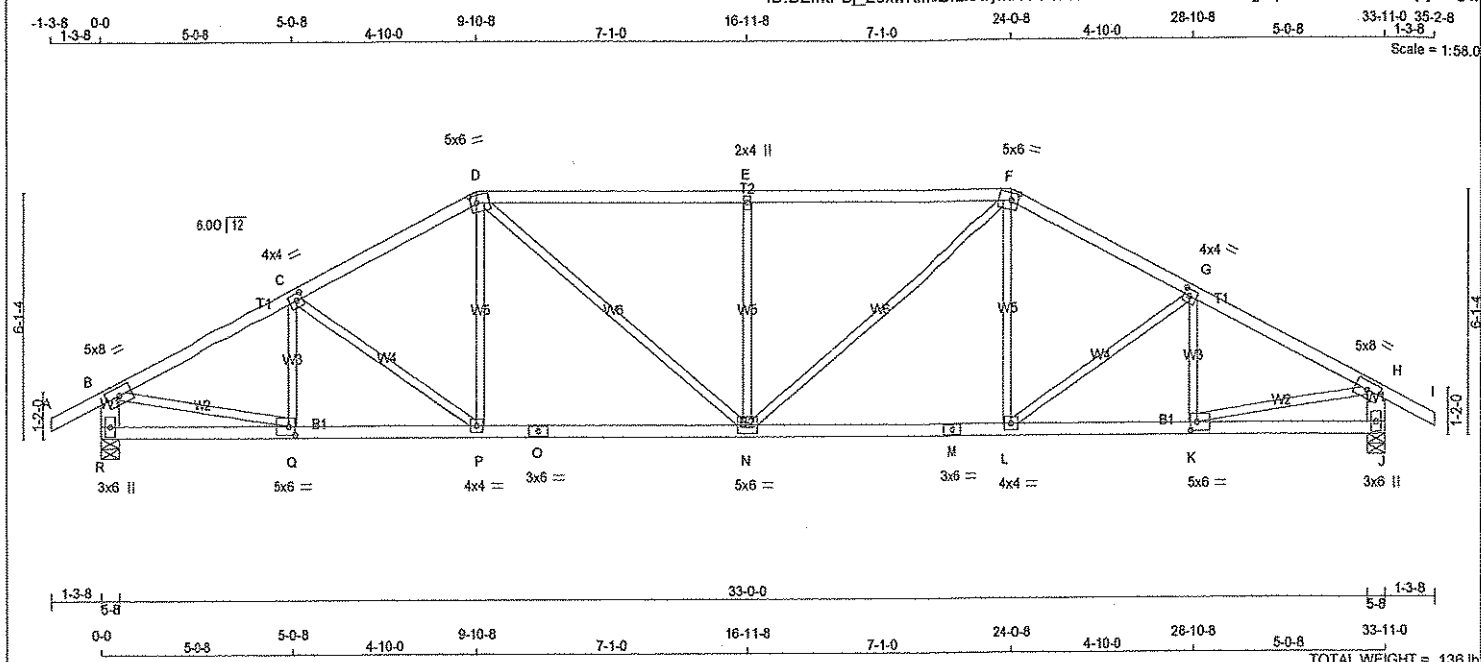
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.69 (L) (INPUT = 0.90)
JSI METAL= 0.72 (I) (INPUT = 1.00)





<u>LUMBER</u>				
N. L. G. A. RULES				
CHORDS	SIZE		LUMBER	DESCR
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
DRY; SEASONED LUMBER.				

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TKW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
H	TMVW-t	MT20	5.0	8.0		
J	BMV1+p	MT20	3.0	6.0		
K	BMWW-t	MT20	5.0	6.0	2.50	2.00
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	5.0	6.0		
O	BS-t	MT20	3.0	6.0		
P	BMWW-t	MT20	4.0	4.0		
Q	BMWW-t	MT20	5.0	6.0	2.50	2.00
R	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER						
<u>BEARINGS</u>						
	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQ'D BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT IN-SX	IN-SX
R	2018	0	2018	0	0	5-8
J	2018	0	2018	0	0	5-8

UNFACTORED REACTIONS							
1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1641	937 / 0	358 / 0	0 / 0	0 / 0	347 / 0	0 / 0
J	1641	937 / 0	358 / 0	0 / 0	0 / 0	347 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.34 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED			
MEMB.	FORCE	VERT. LOAD	LC1 MAX	MAX.	MEMB.	FORCE	MAX
	(LBS)	(PLF)	CSI (LC)	UNBRAC		(LBS)	CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
A-B	0/26	-84.3	-84.3 0.11 (1)	10.00	Q-C	-289/142	0.06 (1)
B-C	-2770/0	-84.3	-84.3 0.37 (1)	3.88	C-P	-228/0	0.14 (1)
C-D	-2603/0	-84.3	-84.3 0.34 (1)	4.60	P-D	0/376	0.08 (2)
D-E	-2883/0	-84.3	-84.3 0.73 (1)	3.34	D-N	0/738	0.17 (1)
E-F	-2883/0	-84.3	-84.3 0.73 (1)	3.34	N-E	-734/0	0.43 (1)
F-G	-2603/0	-84.3	-84.3 0.34 (1)	4.60	N-F	0/738	0.17 (1)
G-H	-2770/0	-84.3	-84.3 0.37 (1)	3.88	L-F	0/376	0.08 (2)
H-I	0/26	-84.3	-84.3 0.11 (1)	10.00	L-G	-228/0	0.14 (1)
R-B	-1954/0	0.0	0.0 0.13 (1)	7.20	K-G	-289/142	0.06 (1)
J-H	-1954/0	0.0	0.0 0.13 (1)	7.20	B-Q	0/2532	0.57 (1)
					K-H	0/2532	0.57 (1)
R-Q	0/0	-28.0	-28.0 0.15 (2)	10.60			
Q-P	0/2496	-28.0	-28.0 0.52 (1)	10.60			
P-O	0/2313	-28.0	-28.0 0.56 (2)	10.60			
O-N	0/2313	-28.0	-28.0 0.56 (2)	10.60			
N-M	0/2313	-28.0	-28.0 0.56 (2)	10.60			
M-L	0/2313	-28.0	-28.0 0.56 (2)	10.60			
L-K	0/2496	-28.0	-28.0 0.52 (1)	10.60			
K-J	0/0	-28.0	-28.0 0.15 (2)	10.60			

DESIGN CRITERIA	
-----------------	--

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	46.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBG 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= $L/360$ (1.13")
 CALCULATED VERT. DEFL.(LL) = $L/999$ (0.19")
 ALLOWABLE DEFL.(TL) = $L/360$ (1.13")
 CALCULATED VERT. DEFL.(TL) = $L/999$ (0.32")

CSI: TC=0.73 (D-E:1), BC=0.56 (L-N:2), WB=0.57 (H-K:1), SSI=0.29 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR ≈ 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

	NAIL VALUES		GRIP (DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656		

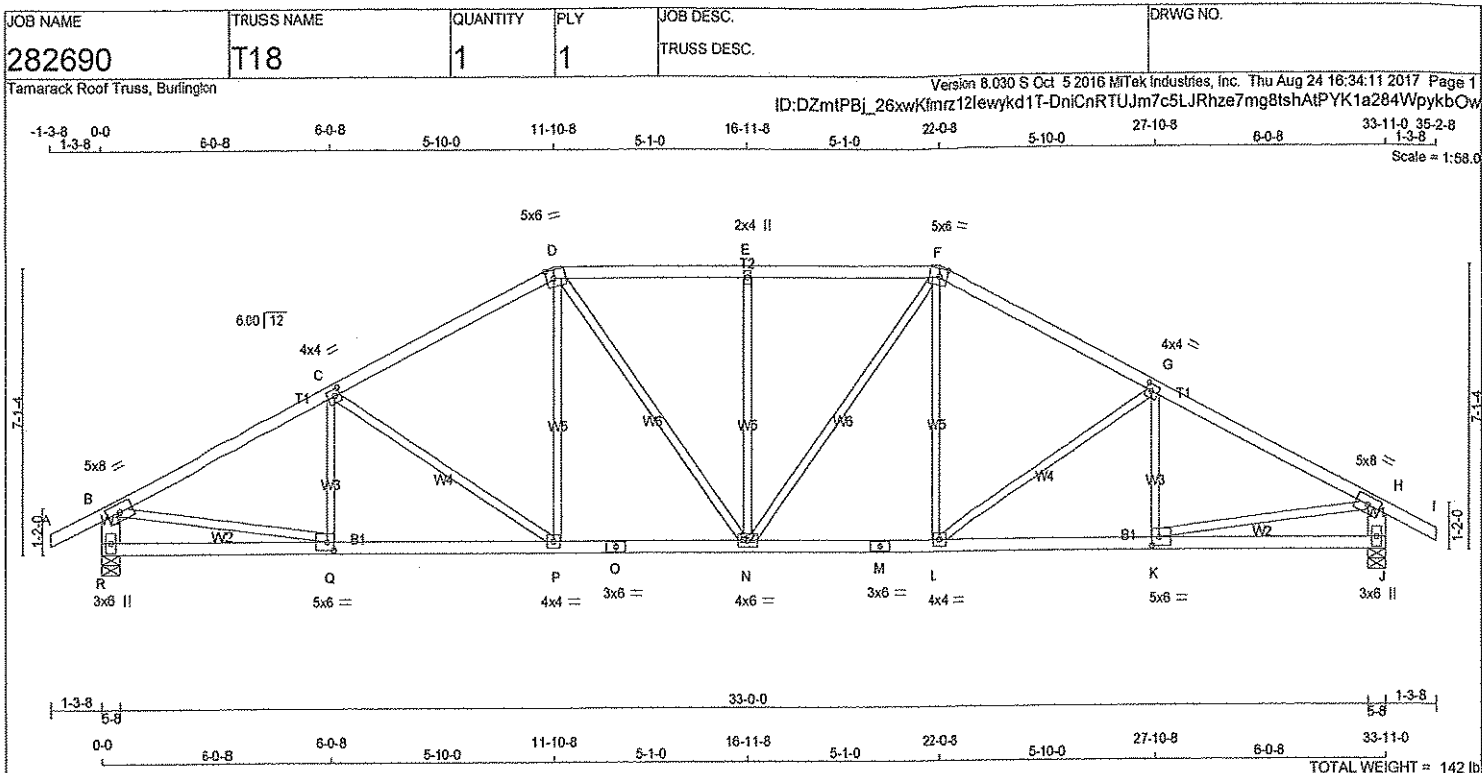
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (D) (INPUT = 0.90)
JSI METAL= 0.62 (K) (INPUT = 1.00)



DWG NO. TAM 4287411
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2 SPF
D - F	2x4	DRY	No.2 SPF
F - I	2x4	DRY	No.2 SPF
R - B	2x6	DRY	No.2 SPF
J - H	2x6	DRY	No.2 SPF
R - O	2x4	DRY	No.2 SPF
O - M	2x4	DRY	No.2 SPF
M - J	2x4	DRY	No.2 SPF
ALL WEBS EXCEPT	2x3	DRY	No.2 SPF

DRY: SEASONED LUMBER

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0		
C	TMVW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	6.0	2.50	2.25
E	TMVW-t	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.50	2.25
G	TMVW-t	MT20	4.0	4.0	2.00	1.75
H	TMVW-t	MT20	5.0	8.0		
J	BMV1+p	MT20	3.0	6.0		
K	BMVW-t	MT20	5.0	6.0	2.50	2.25
L	BMVW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMVW-t	MT20	4.0	6.0		
O	BS-t	MT20	3.0	6.0		
P	BMVW-t	MT20	4.0	4.0		
Q	BMVW-t	MT20	5.0	6.0	2.50	2.25
R	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	UPLIFT	IN-SX
R	2018	0	0	5-8
J	2018	0	0	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX/MIN COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	COMBINED							
R	1641	937 / 0	356 / 0	0 / 0	0 / 0	347 / 0	0 / 0	0 / 0
J	1641	937 / 0	356 / 0	0 / 0	0 / 0	347 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.67 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED HORIZ. LOAD (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED HORIZ. LOAD (LC)
FR-TO				FR-TO			
A-B	0 / 26	-84.3	-84.3 0.11 (1)	O-C	-179 / 152	0.05 (1)	
B-C	-2821 / 0	-84.3	-84.3 0.54 (1)	C-P	-458 / 0	0.44 (1)	
C-D	-2448 / 0	-84.3	-84.3 0.48 (1)	P-D	0 / 438	0.10 (2)	
D-E	-2413 / 0	-84.3	-84.3 0.35 (1)	D-N	0 / 406	0.09 (1)	
E-F	-2413 / 0	-84.3	-84.3 0.35 (1)	N-E	-521 / 0	0.46 (1)	
F-G	-2448 / 0	-84.3	-84.3 0.48 (1)	N-F	0 / 406	0.09 (1)	
G-H	-2821 / 0	-84.3	-84.3 0.54 (1)	L-F	0 / 438	0.10 (2)	
H-I	0 / 26	-84.3	-84.3 0.11 (1)	L-G	-458 / 0	0.44 (1)	
R-B	-1947 / 0	0.0	0.0 0.13 (1)	K-G	-179 / 152	0.05 (1)	
J-H	-1947 / 0	0.0	0.0 0.13 (1)	B-Q	0 / 2572	0.58 (1)	
				K-H	0 / 2572	0.58 (1)	
R-Q	0 / 0	-28.0	-28.0 0.24 (3)				
Q-P	0 / 2546	-28.0	-28.0 0.53 (2)				
P-O	0 / 2170	-28.0	-28.0 0.44 (1)				
O-N	0 / 2170	-28.0	-28.0 0.44 (1)				
N-M	0 / 2170	-28.0	-28.0 0.44 (1)				
M-L	0 / 2170	-28.0	-28.0 0.44 (1)				
L-K	0 / 2546	-28.0	-28.0 0.53 (2)				
K-J	0 / 0	-28.0	-28.0 0.24 (3)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (1.13")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.16")
ALLOWABLE DEFL.(TL) = $L/360$ (1.13")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.26")

CSI: TC=0.54 (G-H:1), BC=0.53 (P-Q:2), WB=0.58 (B-Q:1), SS=0.22 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

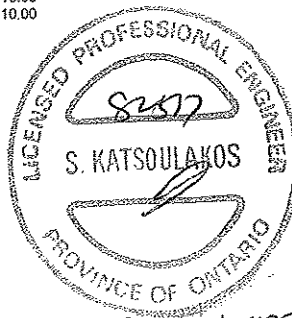
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL = 0.64 (Q) (INPUT = 1.00)



DWG NO. TAM 42875-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
282690	T19	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 Mitek Industries, Inc. Thu Aug 24 16:34:12 2017 Page 2

ID:DZmtPBj 26xwKfmrz12lewykd!T-h Ga mU644FTVudFg9Muh5g5SW8yiBpild2FykbOv

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
O	BMW+w	MT20	3.0	10.0		
P	BS-I	MH16	5.0	17.5		
Q	BMW+W-I	MT20	8.0	9.0		
R	BMVW1-I	MT20	10.0	12.0	Edge 5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1538.9 lbs FACTORED DOWN AT 2-0-12, 1538.9 lbs FACTORED DOWN AT 4-0-12, 1538.9 lbs FACTORED DOWN AT 6-0-12, 1538.9 lbs FACTORED DOWN AT 8-0-12, 1538.9 lbs FACTORED DOWN AT 9-0-4, 1623.1 lbs FACTORED DOWN AT 10-0-12, 1623.1 lbs FACTORED DOWN AT 12-0-12, 1623.1 lbs FACTORED DOWN AT 14-0-12, 1623.1 lbs FACTORED DOWN AT 16-0-12, 1623.1 lbs FACTORED DOWN AT 17-6-4, 1623.1 lbs FACTORED DOWN AT 19-6-4, 1623.1 lbs FACTORED DOWN AT 21-6-4, 1623.1 lbs FACTORED DOWN AT 23-6-4, 1623.1 lbs FACTORED DOWN AT 25-6-4, 1623.1 lbs FACTORED DOWN AT 27-6-4, 1623.1 lbs FACTORED DOWN AT 29-6-4, AND 1623.1 lbs FACTORED DOWN AT 31-6-4, AND 509.9 lbs FACTORED DOWN AT 31-10-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

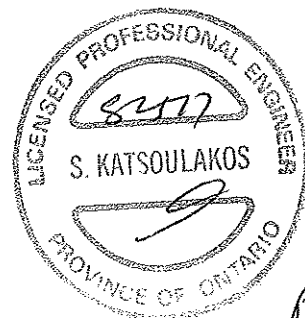
LOADING

TOTAL LOAD CASES: (4)

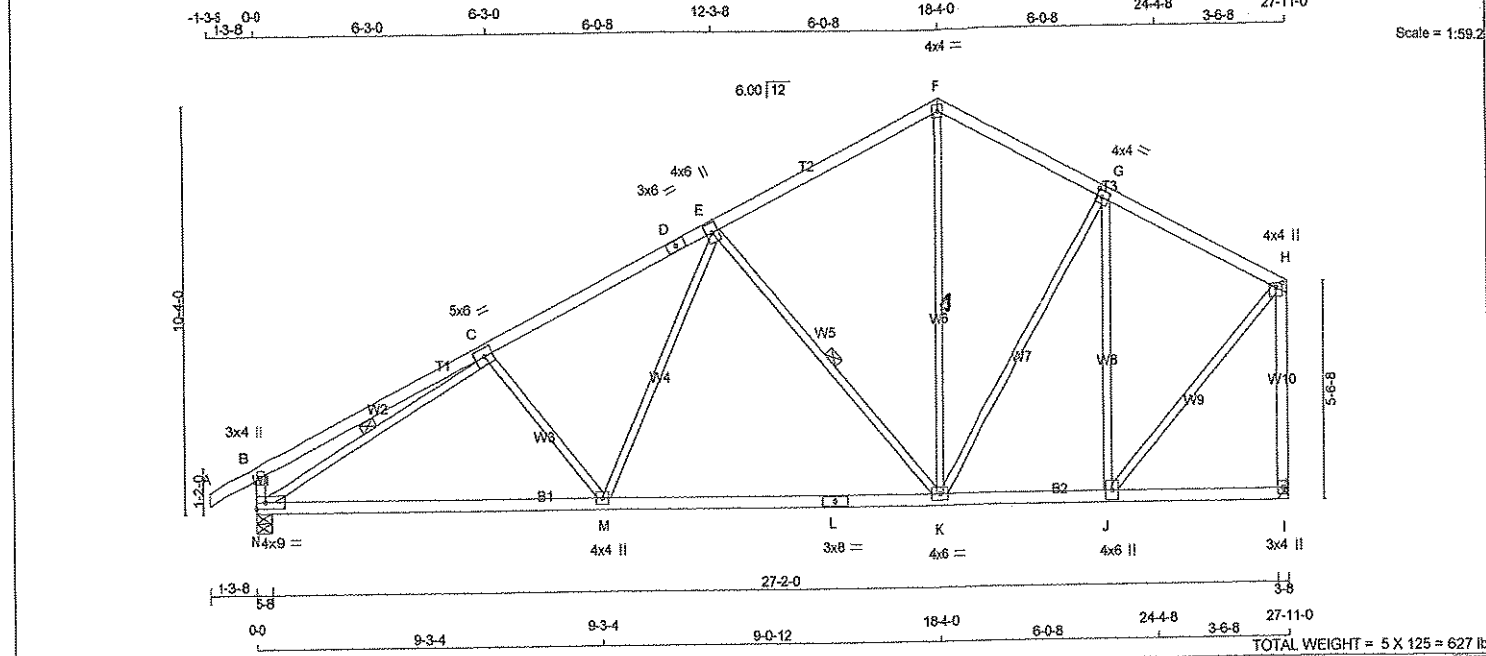
CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 (LC)	MAX. UNBRAC LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO		FROM	TO				
AD-AE	0 / 25892	-28.0	-28.0	0.76 (1)	10.00		
AE-K	0 / 25892	-28.0	-28.0	0.76 (1)	10.00		
K-AF	0 / 16913	-28.0	-28.0	0.62 (1)	10.00		
AF-AG	0 / 16913	-28.0	-28.0	0.62 (1)	10.00		
AG-AH	0 / 16913	-28.0	-28.0	0.62 (1)	10.00		
AH-J	0 / 16913	-28.0	-28.0	0.62 (1)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
P	9-9-4	-1539	-1539	---	BACK	VERT	TOTAL
Q	6-0-12	-1539	-1539	---	BACK	VERT	TOTAL
S	2-0-12	-1539	-1539	---	BACK	VERT	TOTAL
T	4-0-12	-1539	-1539	---	BACK	VERT	TOTAL
U	8-0-12	-1539	-1539	---	BACK	VERT	TOTAL
V	10-0-12	-1623	-1623	---	BACK	VERT	TOTAL
W	12-0-12	-1623	-1623	---	BACK	VERT	TOTAL
X	14-0-12	-1623	-1623	---	BACK	VERT	TOTAL
Y	16-0-12	-1623	-1623	---	BACK	VERT	TOTAL
Z	17-6-4	-1623	-1623	---	BACK	VERT	TOTAL
AA	19-6-4	-1623	-1623	---	BACK	VERT	TOTAL
AB	21-6-4	-1623	-1623	---	BACK	VERT	TOTAL
AC	23-6-4	-1623	-1623	---	BACK	VERT	TOTAL
AD	25-6-4	-1623	-1623	---	BACK	VERT	TOTAL
AE	27-6-4	-1623	-1623	---	BACK	VERT	TOTAL
AF	29-6-4	-1623	-1623	---	BACK	VERT	TOTAL
AG	31-6-4	-1623	-1623	---	BACK	VERT	TOTAL
AH	31-10-4	-510	-510	---	BACK	VERT	TOTAL



DWG NO. TAM 4206 17
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 5 X 125 = 627 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-I	MT20	5.0	6.0		
D	TS-I	MT20	3.0	6.0		
E	TMWW+I	MT20	4.0	6.0		
F	TTW-p	MT20	4.0	4.0		
G	TMWW-I	MT20	4.0	4.0	2.00	1.75
H	TMVW+p	MT20	4.0	4.0	1.25	2.00
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-I	MT20	4.0	6.0		
K	BMWW-I	MT20	4.0	6.0		
L	BS-I	MT20	3.0	8.0		
M	BMWW+I	MT20	4.0	4.0		
N	BMVW1-I	MT20	4.0	9.0		Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
N	1681	0	1681	0	5-8	5-8
I	1567	0	1567	0	5-8	5-8

HANGER BY OTHERS
MIN. SEAT SIZE: 3-8

UNFACTORED REACTIONS

JT	1ST CASE		MAX/MIN COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
N	1364	784 / 0	293 / 0	0 / 0	0 / 0	287 / 0	0 / 0
I	1287	714 / 0	293 / 0	0 / 0	0 / 0	279 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.39 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-K, C-N. *E-K*

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 26	-84.3 -84.3	0.11 (1)	C-M	-268 / 74	0.11 (1)	
B-C	0 / 22	-84.3 -84.3	0.41 (1)	M-E	0 / 591	0.13 (2)	
C-D	-2002 / 0	-84.3 -84.3	0.41 (1)	E-K	-889 / 0	0.54 (1)	
D-E	-2002 / 0	-84.3 -84.3	0.41 (1)	K-F	0 / 643	0.14 (1)	
E-F	-1135 / 0	-84.3 -84.3	0.38 (1)	F-G	0 / 262	0.08 (1)	
F-G	-1135 / 0	-84.3 -84.3	0.26 (1)	G-H	-791 / 0	0.97 (1)	
G-H	-939 / 0	-84.3 -84.3	0.25 (1)	H-I	-2324 / 0	0.82 (1)	
H-I	-325 / 0	0.0 0.0	0.03 (1)	I-J	0 / 1259	0.28 (1)	
I-H	-1508 / 0	0.0 0.0	0.88 (1)				
N-M	0 / 1959	-28.0 -28.0	0.84 (2)				
M-L	0 / 1577	-28.0 -28.0	0.80 (2)				
L-K	0 / 1577	-28.0 -28.0	0.80 (2)				
K-J	0 / 859	-28.0 -28.0	0.38 (2)				
J-I	0 / 0	-28.0 -28.0	0.14 (2)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	PSF
	25.6	PSF
BOT CH.	LL	PSF
	10.5	PSF
	7.0	PSF
TOTAL LOAD	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.93")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.28")
ALLOWABLE DEFL.(TL) = $L/360$ (0.93")
CALCULATED VERT. DEFL.(TL) = $L/727$ (0.46")

CSI: TC=0.88 (H-I:1), BC=0.84 (M-N:2), WB=0.97 (G-J:1), SSI=0.22 (M-N:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

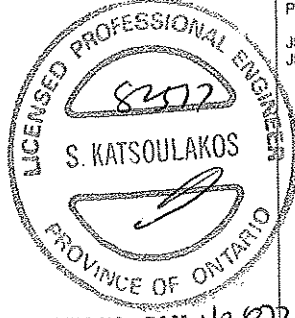
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (M) (INPUT = 0.90)
JSI METAL= 0.62 (L) (INPUT = 1.00)



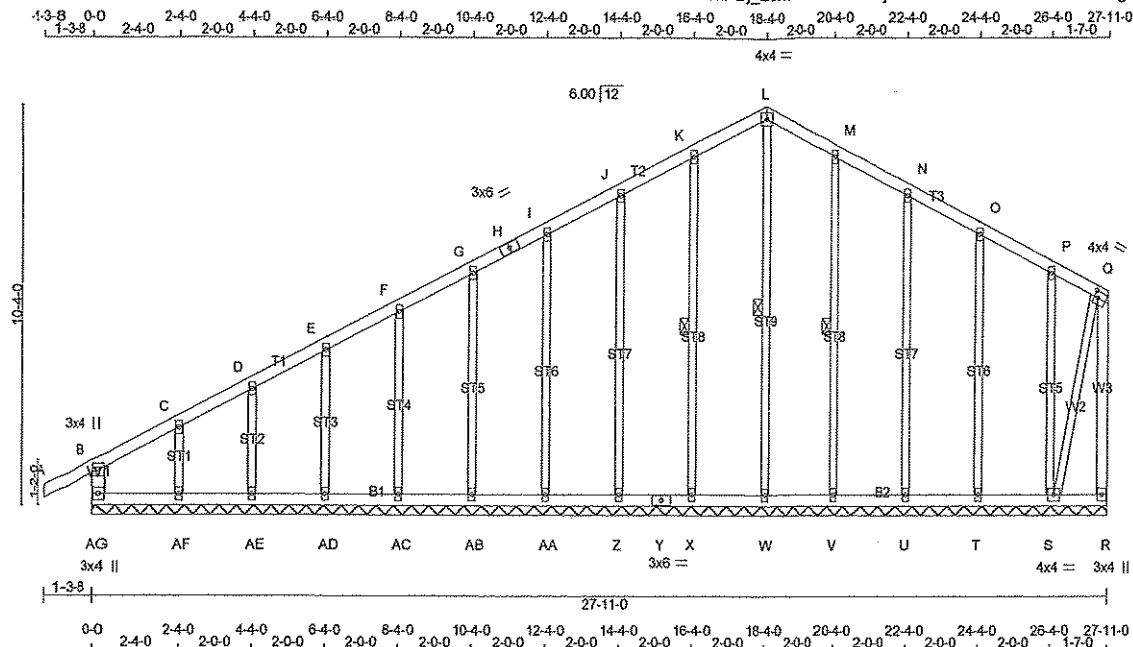
DRWNO. TAM 42872-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
282690	G20	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Thu Aug 24 16:34:09 2017 Page 1

ID:DZmtPBj_26wKfmrz12lewykd1T-HPaSMISEn9tur2A2aYcF3e3tc7xkLk6fzRwykbOy



Scale = 1:60.3

TOTAL WEIGHT = 146 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - H	2x4	DRY	No.2
H - L	2x4	DRY	No.2
L - Q	2x4	DRY	No.2
AG - B	2x4	DRY	No.2
R - Q	2x4	DRY	No.2
AG - Y	2x4	DRY	No.2
Y - R	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			
GABLE STUDS SPACED AT 2'-0" O.C.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.
1 LATERAL BRACE(S) AT 1/2 LENGTH OF L-W, K-X, M-V.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C, D, E, F, G, I, J, K, M, N, Q, P						
C	TMV+w	MT20	2.0	4.0		
H	TS-I	MT20	3.0	6.0		
L	TTW-p	MT20	4.0	4.0		
Q	TMVW-I	MT20	4.0	4.0	2.00	1.25
R	BMV1+p	MT20	3.0	4.0		
S	BMVW1-I	MT20	4.0	4.0		
T, U, V, W, X, Z, AA, AB, AC, AD, AE, AF						
T	BMV1+w	MT20	2.0	4.0		
Y	BS-I	MT20	3.0	6.0		
AG	BMV1+p	MT20	3.0	4.0		

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0/28	-84.3 -84.3 0.11 (1)	10.00	W-L	-161/0	0.12 (1)	
B-C	0/8	-84.3 -84.3 0.06 (1)	10.00	X-K	-191/0	0.11 (1)	
C-D	0/4	-84.3 -84.3 0.06 (1)	10.00	Z-J	-163/0	0.23 (1)	
D-E	0/12	-84.3 -84.3 0.04 (1)	10.00	AA-I	-168/0	0.18 (1)	
E-F	0/14	-84.3 -84.3 0.04 (1)	10.00	AB-G	-168/0	0.11 (1)	
F-G	0/17	-84.3 -84.3 0.04 (1)	10.00	AC-F	-167/0	0.07 (1)	
G-H	0/19	-84.3 -84.3 0.04 (1)	10.00	AD-E	-169/0	0.05 (1)	
H-I	0/19	-84.3 -84.3 0.04 (1)	10.00	AE-D	-157/0	0.03 (1)	
I-J	0/20	-84.3 -84.3 0.04 (1)	10.00	AF-C	-203/0	0.03 (1)	
J-K	0/24	-84.3 -84.3 0.05 (1)	10.00	V-M	-192/0	0.11 (1)	
K-L	0/16	-84.3 -84.3 0.05 (1)	10.00	U-N	-163/0	0.22 (1)	
L-M	0/16	-84.3 -84.3 0.05 (1)	10.00	T-O	-172/0	0.17 (1)	
M-N	0/24	-84.3 -84.3 0.05 (1)	10.00	S-P	-159/0	0.10 (1)	
N-O	0/20	-84.3 -84.3 0.04 (1)	10.00	S-Q	-52/0	0.03 (1)	
O-P	0/20	-84.3 -84.3 0.04 (1)	10.00				
P-Q	0/23	-84.3 -84.3 0.03 (1)	10.00				
AG-B	-192/0	0.0 0.0 0.02 (1)	7.81				
R-Q	0/8	0.0 0.0 0.00 (1)	10.00				
AG-AF	0/0	-28.0 -28.0 0.04 (2)	10.00				
AF-AE	-7/0	-28.0 -28.0 0.04 (2)	10.00				
AE-AD	-10/0	-28.0 -28.0 0.02 (2)	6.25				
AD-AC	-13/0	-28.0 -28.0 0.02 (2)	6.25				
AC-AB	-15/0	-28.0 -28.0 0.02 (2)	6.25				
AB-AA	-17/0	-28.0 -28.0 0.02 (2)	6.25				
AA-Z	-18/0	-28.0 -28.0 0.02 (2)	6.25				
Z-Y	-20/0	-28.0 -28.0 0.02 (2)	6.25				
Y-X	-20/0	-28.0 -28.0 0.02 (2)	6.25				
X-W	-21/0	-28.0 -28.0 0.02 (2)	6.25				
W-V	-21/0	-28.0 -28.0 0.02 (2)	6.25				
V-U	-20/0	-28.0 -28.0 0.02 (2)	6.25				
U-T	-18/0	-28.0 -28.0 0.02 (2)	6.25				
T-S	-17/0	-28.0 -28.0 0.02 (2)	6.25				
S-R	0/0	-28.0 -28.0 0.02 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11 (A-B:1), BC=0.04 (AE-AF:2), WB=0.23 (J-Z:1), SS=0.08 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

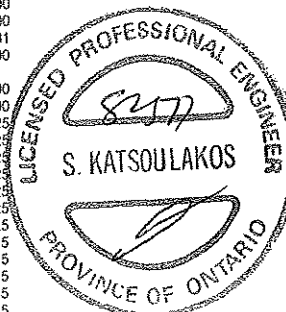
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

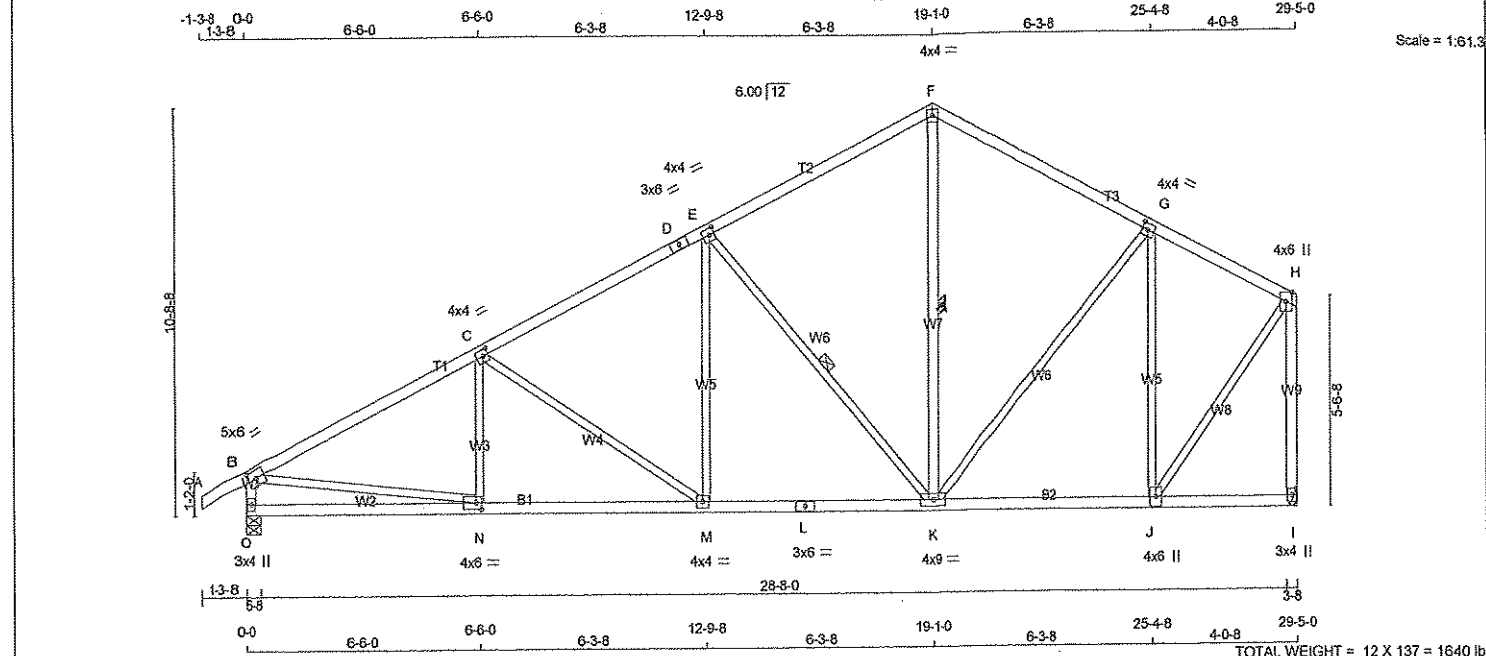
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.52 (S) (INPUT = 0.90)
JSI METAL= 0.93 (C) (INPUT = 1.00)



DWG NO. TAM 42682-17
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 12 X 137 = 1640 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2 SPF
D - F	2x4	DRY	No.2 SPF
F - H	2x4	DRY	No.2 SPF
O - B	2x4	DRY	No.2 SPF
I - H	2x4	DRY	No.2 SPF
O - L	2x4	DRY	No.2 SPF
L - I	2x4	DRY	No.2 SPF
ALL WEBS EXCEPT K - F	2x3	DRY	No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-I	MT20	5.0	6.0	2.50	2.75
C, E, G						
C	TMVW-I	MT20	4.0	4.0	2.00	1.75
D	TS-I	MT20	3.0	6.0		
F	TTW-p	MT20	4.0	4.0		
H	TMVW+p	MT20	4.0	6.0	Edge	
I	BMV1+p	MT20	3.0	4.0		
J	BMVW+I	MT20	4.0	6.0		
K	BMVW+I	MT20	4.0	9.0		
L	BS-I	MT20	3.0	6.0		
M	BMVW-I	MT20	4.0	4.0		
N	BMVW-I	MT20	4.0	6.0	2.00	1.75
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
O	1765	0	1765	0	5-8	5-8
I	1651	0	1651	0		HANGER BY OTHERS MIN. SEAT SIZE: 3-8

UNFACTORED REACTIONS							
JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
O	1433	822 / 0	309 / 0	0 / 0	0 / 0	362 / 0	0 / 0
I	1355	753 / 0	309 / 0	0 / 0	0 / 0	294 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.94 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-K. *E-K*

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
 TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS1 (LC)	
FR-TO				FR-TO			
A-B	0 / 26	-84.3	-84.3 0.11 (1)	N-C	-84 / 212	0.05 (3)	
B-C	-2370 / 0	-84.3	-84.3 0.56 (1)	C-M	-524 / 0	0.59 (1)	
C-D	-1902 / 0	-84.3	-84.3 0.43 (1)	M-E	0 / 507	0.11 (2)	
D-E	-1902 / 0	-84.3	-84.3 0.43 (1)	E-K	-958 / 0	0.62 (1)	
E-F	-1224 / 0	-84.3	-84.3 0.42 (1)	K-F	0 / 645	0.10 (1)	
F-G	-1224 / 0	-84.3	-84.3 0.42 (1)	K-G	0 / 348	0.08 (1)	
G-H	-912 / 0	-84.3	-84.3 0.33 (1)	J-G	-938 / 0	0.98 (1)	
O-B	-1690 / 0	0.0	0.0 0.17 (1)	B-N	0 / 2158	0.49 (1)	
I-H	-1612 / 0	0.0	0.0 0.94 (1)	J-H	0 / 1384	0.31 (1)	
O-N	0 / 0	-28.0	-28.0 0.28 (3)				
N-M	0 / 2140	-28.0	-28.0 0.52 (2)				
M-L	0 / 1701	-28.0	-28.0 0.43 (2)				
L-K	0 / 1701	-28.0	-28.0 0.43 (2)				
K-J	0 / 846	-28.0	-28.0 0.33 (2)				
J-I	0 / 0	-28.0	-28.0 0.18 (3)				

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 25.6 PSF
 DL = 3.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 088-09
 - TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.98")
 CALCULATED VERT. DEFL. (LL) = L/999 (0.11")
 ALLOWABLE DEFL. (TL) = L/360 (0.98")
 CALCULATED VERT. DEFL. (TL) = L/999 (0.19")

CSI: TC=0.94 (H-I:1), BC=0.52 (M-N:2), WB=0.98 (G-J:1), SSI=0.23 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

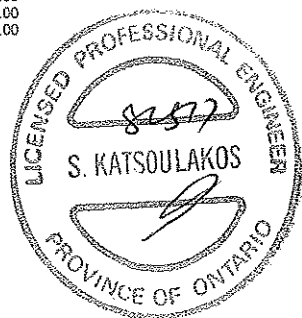
NAIL VALUES

PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
MT20	618	354	1687
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (D) (INPUT = 0.90)
 JSI METAL= 0.64 (B) (INPUT = 1.00)



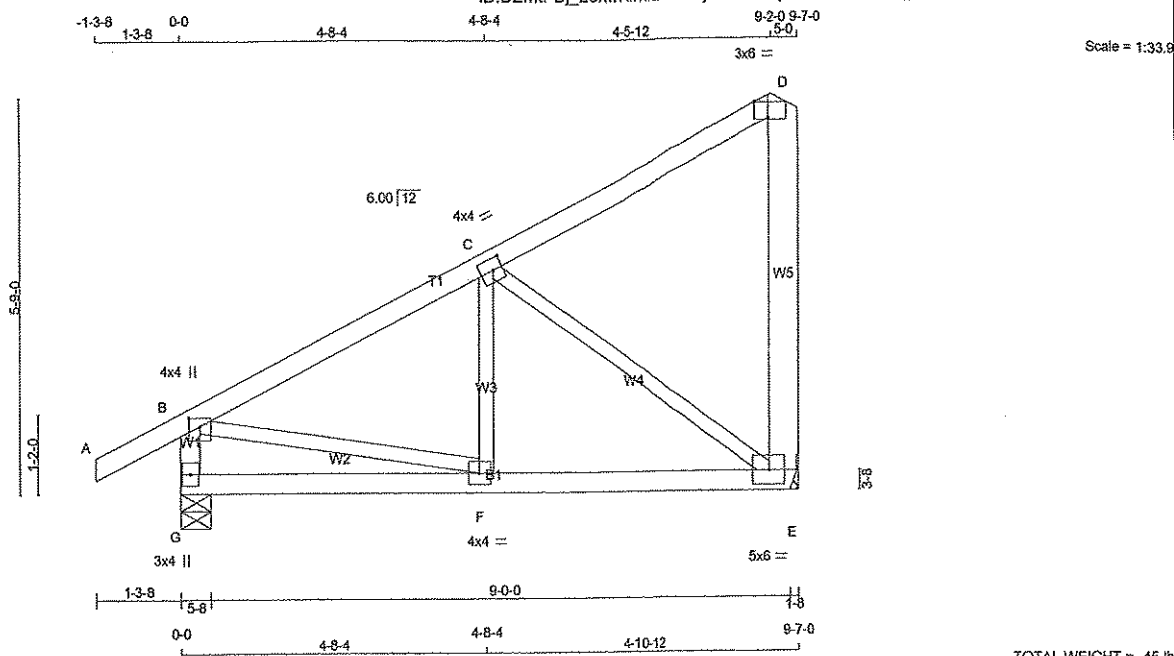
DWG NO. TAM 4287E-17
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
282690	T22	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 6.030 S Oct 5 2016 Mitek Industries, Inc. Thu Aug 24 16:34:13 2017 Page 1

ID: DZmtPBj_26xwKfmz12lewykd1T-9AqzB6VkrONKfTqpOhbr5DhtUwutX0K1MdAaiykbOu



TOTAL WEIGHT = 45 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
G - B	2x4	DRY	No.2	SPF	
E - D	2x6	DRY	No.2	SPF	
G - E	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.50	2.00
C	TMVW-t	MT20	4.0	4.0	2.00	1.75
D	TMV-p	MT20	3.0	6.0	Edge	2.50
E	BMVW-t	MT20	5.0	6.0		
F	BMVW-t	MT20	4.0	4.0		
G	BMV-t+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
G	652	0	652	0
E	538	0	538	0

MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS	DEAD	SOIL
JT	COMBINED		
G	519	315 / 0	101 / 0
E	442	245 / 0	101 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1 (PLF)	MEMB.	FORCE (LBS)
FR-TO		FROM TO	FR-TO	
A-B	0 / 26	-84.3 -84.3 0.11 (1)	F-C	0 / 245
B-C	-533 / 0	-84.3 -84.3 0.25 (1)	B-F	0 / 505
C-D	-21 / 0	-84.3 -84.3 0.24 (1)	C-E	-594 / 0
G-B	-601 / 0	0.0 0.0 0.06 (1)		
E-D	-159 / 0	0.0 0.0 0.07 (1)		
G-F	0 / 0	-28.0 -28.0 0.18 (3)		
F-E	0 / 497	-28.0 -28.0 0.24 (2)		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.32")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.32")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.25 (B-C:1), BC=0.24 (E-F:2), WB=0.31 (C-E:1), SSI=0.18 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.63 (B) (INPUT = 0.90)
JSI METAL= 0.19 (F) (INPUT = 1.00)



DWG NO. TAM 42819-17

STRUCTURAL

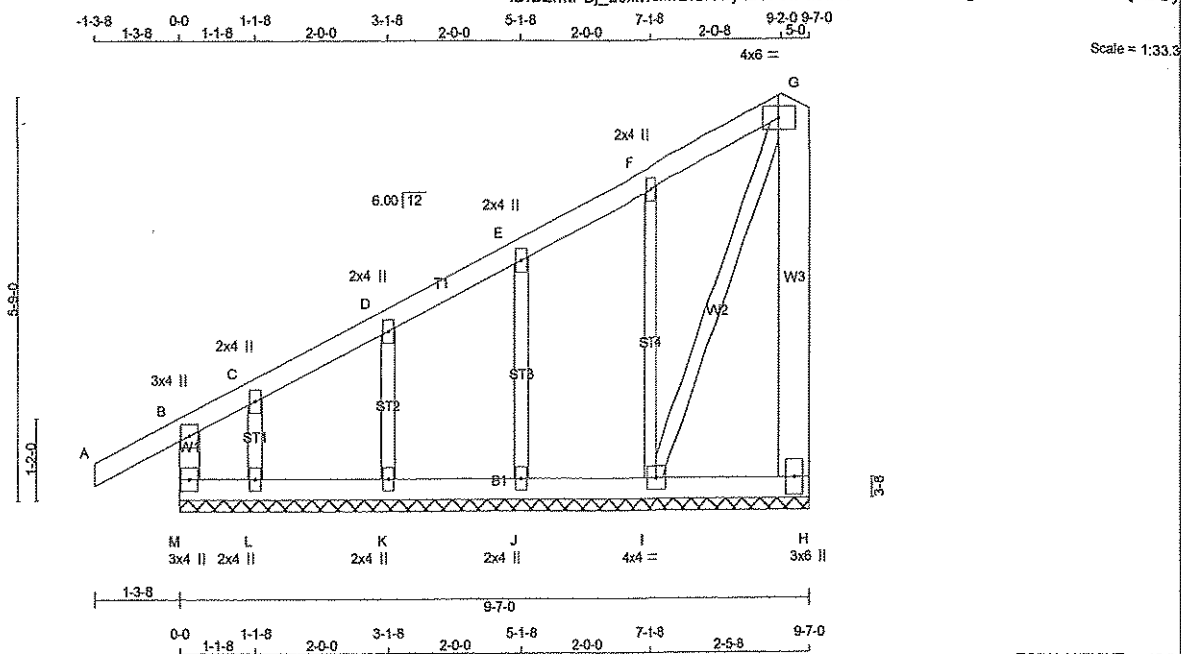
CONSTRUCTION

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
282690	G22	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 5 Oct 5 2016 MTek Industries, Inc. Thu Aug 24 16:34:09 2017 Page 1

ID:DZmtPBj_26xwKmrz12lwykd1T-HPaSMISEn9tur2A2aYcFgF3e3tc4xmkk6lfzRwykbOy



TOTAL WEIGHT = 48 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - G	2x4	DRY	No.2
M - B	2x4	DRY	No.2
H - G	2x6	DRY	No.2
M - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER			
GABLE STUDS SPACED AT 2-0-0 OC.			

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	3.0	4.0
C, D, E, F				
C	TMW+w	MT20	2.0	4.0
G	TVMW+p	MT20	4.0	6.0
H	BMV1+p	MT20	3.0	6.0
I	BMW1+1	MT20	4.0	4.0
J, K, L				
J	BMW1+w	MT20	2.0	4.0
M	BMV1+p	MT20	3.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC1)	UNBRACED LENGTH (FR-TO)
FR-TO				FR-TO			
A-B	0/26	-84.3	-84.3 0.11 (1)	I-F	-220/0	0.07 (1)	
B-C	-29/0	-84.3	-84.3 0.11 (1)	J-E	-150/0	0.03 (1)	
C-D	0/8	-84.3	-84.3 0.05 (1)	K-D	-183/0	0.03 (1)	
D-E	0/6	-84.3	-84.3 0.05 (1)	L-C	-52/0	0.01 (1)	
E-F	0/17	-84.3	-84.3 0.07 (1)	I-G	-34/0	0.02 (1)	
F-G	0/7	-84.3	-84.3 0.07 (1)				
M-B	-223/0	0.0	0.0 0.02 (1)				
H-G	-83/0	0.0	0.0 0.03 (1)				
M-L	0/0	-28.0	-28.0 0.02 (2)				
L-K	-2/0	-28.0	-28.0 0.02 (2)				
K-J	-7/0	-28.0	-28.0 0.02 (2)				
J-I	-10/0	-28.0	-28.0 0.04 (2)				
I-H	0/0	-28.0	-28.0 0.04 (2)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11 (A-B:1), BC=0.04 (I-J:2), WB=0.07 (F-I:1), SSI=0.09 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

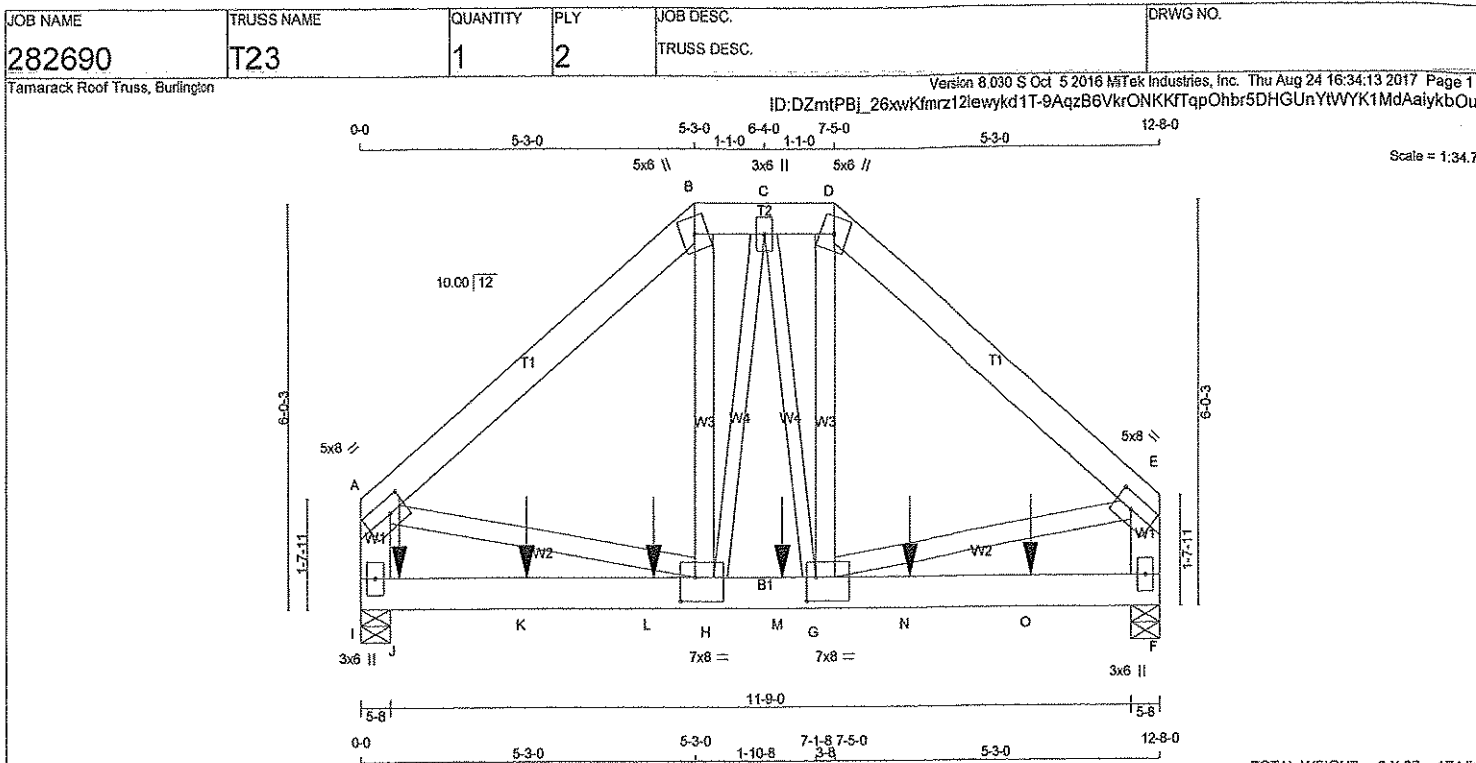
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (F) (INPUT = 0.90)
JSI METAL= 0.05 (F) (INPUT = 1.00)



DRWG NO. TAM 42003 17
STRUCTURAL
COMPONENT ONLY



LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	DRY	DESCR.	
A - B	2x6	DRY	No.2	SPF
B - D	2x6	DRY	No.2	SPF
D - E	2x6	DRY	No.2	SPF
I - A	2x6	DRY	No.2	SPF
F - E	2x6	DRY	No.2	SPF
I - F	2x6	DRY	1650F 1.5E	SPF

ALL WEBS	2x4	DRY	No.2	SPF
EXCEPT				
H - C	2x3	DRY	No.2	SPF
C - G	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122'X3") SPIRAL NAILS			
A - B	2	12	TOP
B - D	2	12	TOP
D - E	2	12	TOP
I - A	2	2	SIDE (27.4)
F - E	2	2	TOP
BOTTOM CHORDS : (0.122'X3") SPIRAL NAILS			
I - F	2	5	SIDE (427.3)
WEBS : (0.122'X3") SPIRAL NAILS			
2x3	1	6	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMWW-t	MT20	5.0	8.0	2.50 3.25
B	TTW+m	MT20	5.0	6.0	
C	TMWW-t	MT20	3.0	6.0	
D	TTW+m	MT20	5.0	6.0	
E	TMWW-l	MT20	5.0	8.0	2.50 3.25
F	BMV1+p	MT20	3.0	6.0	
G	BMWWW-t	MT20	7.0	8.0	4.25 1.75
H	BMWWW-t	MT20	7.0	8.0	4.25 2.75
I	BMV1+p	MT20	3.0	6.0	

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REORD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
I	6529	0	6529	0	0
F	5323	0	5323	0	0

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
I	5288	3074 / 0	1100 / 0	0 / 0	0 / 0	1094 / 0	0 / 0
F	4296	2506 / 0	897 / 0	0 / 0	0 / 0	892 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.11 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD LC1 (LC)	MAX. FACTORED VERT. LOAD LC1 (LC)	MEMB. UNBRACED LENGTH	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD LC1 (LC)
FR-TO									
A-B	-4916 / 0	-84.3	-84.3	0.15 (1)	5.11	H-B	0 / 2301	0.20 (1)	
B-C	-3884 / 0	-84.3	-84.3	0.22 (1)	5.49	G-D	0 / 2185	0.19 (1)	
C-D	-3848 / 0	-84.3	-84.3	0.22 (1)	5.51	A-H	0 / 3841	0.34 (1)	
D-E	-4868 / 0	-84.3	-84.3	0.15 (1)	5.14	G-E	0 / 3803	0.34 (1)	
I-A	-4145 / 0	0.0	0.0	0.15 (1)	7.00	H-C	0 / 685	0.08 (1)	
F-E	-4107 / 0	0.0	0.0	0.15 (1)	7.03	C-G	0 / 515	0.06 (1)	
I-J	0 / 0	-28.0	-28.0	0.67 (1)	10.00				
J-K	0 / 0	-28.0	-28.0	0.67 (1)	10.00				
K-L	0 / 0	-28.0	-28.0	0.67 (1)	10.00				
L-H	0 / 0	-28.0	-28.0	0.67 (1)	10.00				
H-M	0 / 13758	-28.0	-28.0	0.77 (1)	10.00				
M-G	0 / 13758	-28.0	-28.0	0.77 (1)	10.00				
G-N	0 / 0	-28.0	-28.0	0.62 (1)	10.00				
N-O	0 / 0	-28.0	-28.0	0.62 (1)	10.00				
O-F	0 / 0	-28.0	-28.0	0.62 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
JT	7-4	-1743	-1743		FRONT	VERT	TOTAL
K	2-7-4	-1737	-1737		FRONT	VERT	TOTAL
L	4-7-4	-1737	-1737		FRONT	VERT	TOTAL
M	6-7-4	-1737	-1737		FRONT	VERT	TOTAL
N	8-7-4	-1737	-1737		FRONT	VERT	TOTAL
O	10-7-4	-1737	-1737		FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.42")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.12")
ALLOWABLE DEFL. (TL) = $L/360$ (0.42")
CALCULATED VERT. DEFL. (TL) = $L/827$ (0.18")

CSI: TC=0.22 (B-C:1), BC=0.77 (G-H:1), WB=0.34 (A-H:1), SI=0.99 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

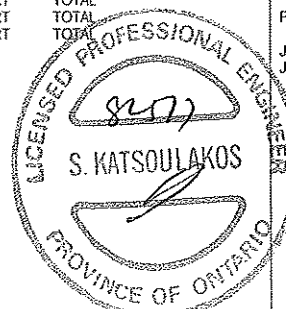
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1657 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (G) (INPUT = 0.90)
JSI METAL= 0.47 (E) (INPUT = 1.00)



DWG NO. TAM 4288017
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
282690	T23	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 Mitek Industries, Inc. Thu Aug 24 16:34:13 2017 Page 2

ID:DZmtPBj_26xwKfmrz12lewykd1T-9AgzB6VkrONKK7TqpOhbr5DHGUnYtVYKIMdAaiykbOu

HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1742.9 lbs FACTORED DOWN AT 7-4, 1737.4 lbs FACTORED DOWN AT 2-7-4, 1737.4 lbs FACTORED DOWN AT 4-7-4, 1737.4 lbs FACTORED DOWN AT 6-7-4, AND 1737.4 lbs FACTORED DOWN AT 8-7-4, AND 1737.4 lbs FACTORED DOWN AT 10-7-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.



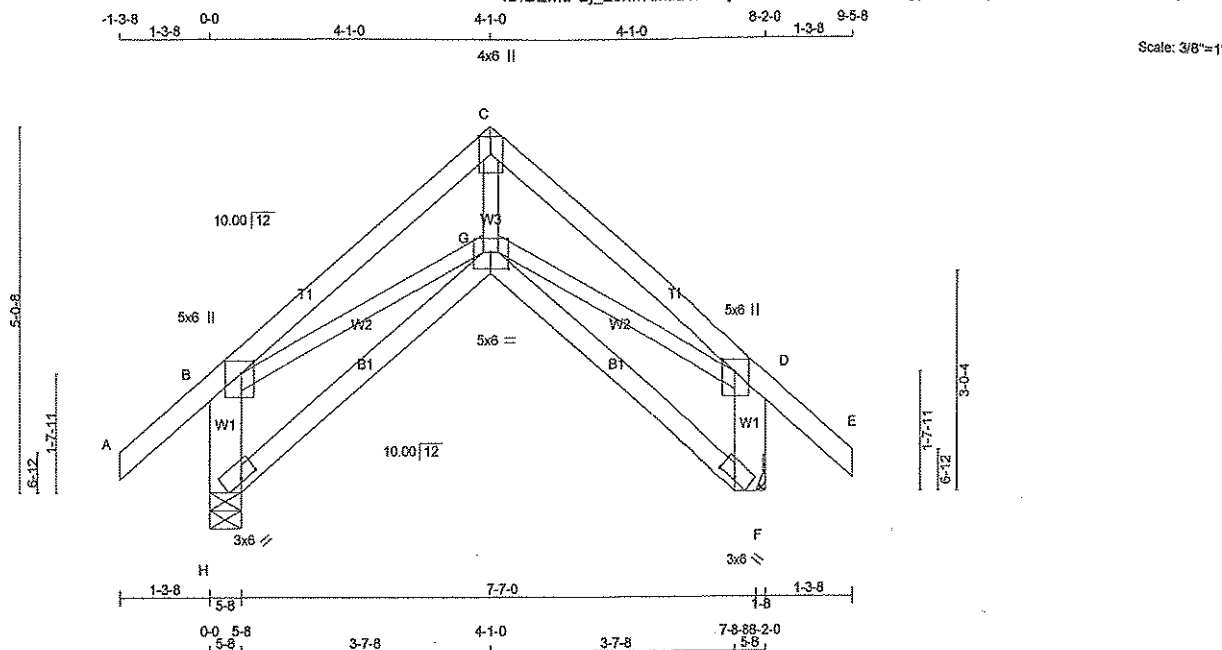
DWG NO. TAM 4188-17
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
282690	T25S	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Thu Aug 24 16:34:14 2017 Page 1

ID:DZmtPBj_26xwKfmrz12lewykd1T-dMOLPSVMchVByp20M5CqNjmtTauHfc0wTG0Mk78ykbO



TOTAL WEIGHT = 3 X 41 = 124 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x6	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMVW+p	MT20	5.0	6.0	2.00	2.25
F	BVM1-I	MT20	3.0	6.0	0.50	3.00
G	BBWWW+p	MT20	5.0	6.0	2.75	3.00
H	BVM1-I	MT20	3.0	6.0	0.50	3.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
H	575	0	575	0
F	575	0	575	0

MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS
JT	COMBINED
H	456
F	456

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE	VERT. LOAD	MEMB.	FORCE
(LBS)	(LBS)	LC1	(LBS)	(LBS)
FR-TO	FROM	TO	FR-TO	FROM
A-B	0/37	-84.3	G-C	0/605
B-C	-741/0	-84.3	G-D	0/620
C-D	-741/0	-84.3	B-G	0/620
D-E	0/37	-84.3		
H-B	-518/0	0.0		
F-D	-518/0	0.0		
H-G	0/0	-28.0		
G-F	0/0	-28.0		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
DL =	3.0	PSF
BOT CH.	LL = 10.5	PSF
DL =	7.0	PSF
TOTAL LOAD =	46.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.27")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.03")
ALLOWABLE DEFL.(TL) = $L/360$ (0.27")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.05")

CSI: TC=0.19 (C-D:1), BC=0.14 (G-H:3), WB=0.14 (D-G:1), SSI=0.10 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)	
	MAX	MIN	MAX	MIN
MT20	618	354	1667	822

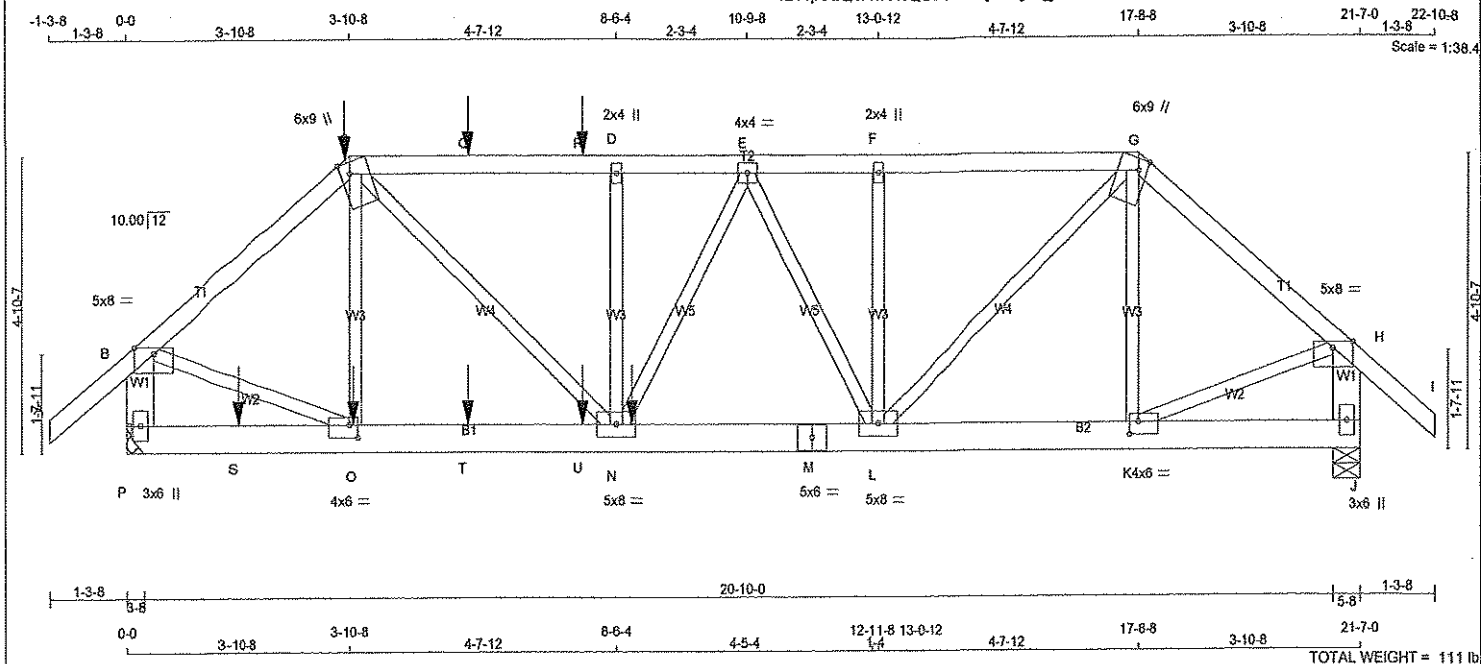
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.51 (G) (INPUT = 0.90)
JSI METAL= 0.16 (B) (INPUT = 1.00)



DWG NO. TAM 4200417
STRUCTURAL
COMPONENT ONLY



N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR
A - C	2x4	DRY No.2	SPF
C - G	2x4	DRY No.2	SPF
G - I	2x4	DRY No.2	SPF
P - B	2x6	DRY No.2	SPF
J - H	2x6	DRY No.2	SPF
P - M	2x6	DRY No.2	SPF
M - J	2x6	DRY No.2	SPF

ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
--------------------	-----	-----	------	-----

DRY: SEASONED LUMBER

PLATES (table 1s in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TTWW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TTWW-w	MT20	2.0	4.0		
E	TTWW-t	MT20	4.0	4.0		
F	TTWW+w	MT20	2.0	4.0		
G	TTWW+m	MT20	6.0	9.0	Edge	1.75
H	TTWW-p	MT20	5.0	8.0	Edge	
J	BMV1+p	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	6.0		2.50
L	BMWWW-t	MT20	5.0	8.0		
M	BS-t	MT20	5.0	6.0		
N	BMWWWW-t	MT20	5.0	8.0		
O	BMWW-t	MT20	4.0	6.0		2.50
P	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 225.2 lbs FACTORED DOWN AT 3-10-8, AND 38.3 lbs FACTORED DOWN AT 5-11-4, AND 38.3 lbs FACTORED DOWN AT 7-11-4 ON TOP CHORD, AND 42.0 lbs FACTORED DOWN AT 1-11-4, 34.1 lbs FACTORED DOWN AT 3-11-4, 34.1 lbs FACTORED DOWN AT 5-11-4, AND 34.1 lbs FACTORED DOWN AT 7-11-4, AND 835.4 lbs FACTORED DOWN AT 8-9-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED GROSS REACTION			MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
P	2135	0	2135	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	
J	1741	0	1741	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1709	1018 / 0	342 / 0	0 / 0	0 / 0	350 / 0	0 / 0
J	1398	827 / 0	284 / 0	0 / 0	0 / 0	287 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.80 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS					
MAX. FACTORED		FACTORED				MAX. FACTORED					
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 (PLF)	MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX. CSI (LC)			
FR-TO		FROM	TO			FR-TO					
A-B	0 / 37	-84.3	-84.3	0.13 (1)	10.00	O-C	-276 / 105	0.09 (1)			
B-C	-2057 / 0	-84.3	-84.3	0.33 (1)	4.37	C-N	0 / 1460	0.35 (1)			
C-Q	-2621 / 0	-84.3	-84.3	0.44 (1)	3.80	N-D	-453 / 0	0.15 (1)			
Q-R	-2621 / 0	-84.3	-84.3	0.44 (1)	3.80	L-F	-391 / 0	0.13 (1)			
R-D	-2621 / 0	-84.3	-84.3	0.44 (1)	3.80	L-G	0 / 1354	0.34 (1)			
D-E	-2621 / 0	-84.3	-84.3	0.35 (1)	3.86	K-G	-236 / 53	0.08 (1)			
E-F	-2208 / 0	-84.3	-84.3	0.29 (1)	4.24	B-O	0 / 1649	0.41 (1)			
F-G	-2208 / 0	-84.3	-84.3	0.33 (1)	4.23	K-H	0 / 1296	0.32 (1)			
G-H	-1617 / 0	-84.3	-84.3	0.30 (1)	4.85	N-E	0 / 443	0.11 (1)			
H-I	0 / 37	-84.3	-84.3	0.13 (1)	10.00	E-L	-473 / 0	0.20 (1)			
P-B	-2086 / 0	0.0	0.0	0.15 (1)	6.99						
J-H	-1790 / 0	0.0	0.0	0.13 (1)	7.58						
P-S	0 / 0	-28.0	-28.0	0.09 (2)	10.00						
S-O	0 / 0	-28.0	-28.0	0.09 (2)	10.00						
O-T	0 / 1570	-28.0	-28.0	0.26 (1)	10.00						
T-U	0 / 1570	-28.0	-28.0	0.26 (1)	10.00						
U-N	0 / 1570	-28.0	-28.0	0.26 (1)	10.00						
N-M	0 / 2422	-28.0	-28.0	0.38 (1)	10.00						
M-L	0 / 2422	-28.0	-28.0	0.38 (1)	10.00						
L-K	0 / 1233	-28.0	-28.0	0.20 (1)	10.00						
K-J	0 / 0	-28.0	-28.0	0.07 (2)	10.00						

LICENSED PROFESSIONAL

82577

S. KATSOULAKI

FACTORED CONCENTRATED LOADS (LBS)

UT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
3-10-8		-225	-225	---	BACK	VERT	TOTAL
CNO	8-9-8	-835	-835	---	BACK	VERT	TOTAL
Q	3-11-4	-20	-34	---	BACK	VERT	TOTAL
R	5-11-4	-38	-38	---	BACK	VERT	TOTAL
Q	7-11-4	-38	-38	---	BACK	VERT	TOTAL
U	1-11-4	-24	-42	---	BACK	VERT	TOTAL
U	5-11-4	-20	-34	---	BACK	VERT	TOTAL
T	7-11-4	-20	-34	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 3.0 PSF

BOT	CH.	CL	=	10.5	PSF
		CL	=	3.0	PSF

	DL	=	7.0	PSF
TOTAL LOAD	is		46.1	PSF

[illegible]

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.72")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.72")
CALCULATED VERT. DEFL.(TL) = L/999 (0.13")

CSI: TC=0.44 (C-D:1), BC=0.38 (L-N:1), WB=0.41 (B-O:1), SS=0.24 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR ≈ 0.50

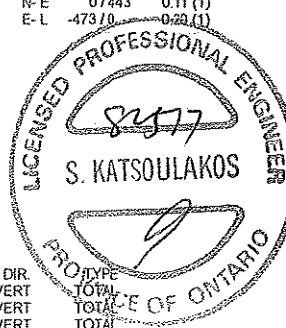
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES						
PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

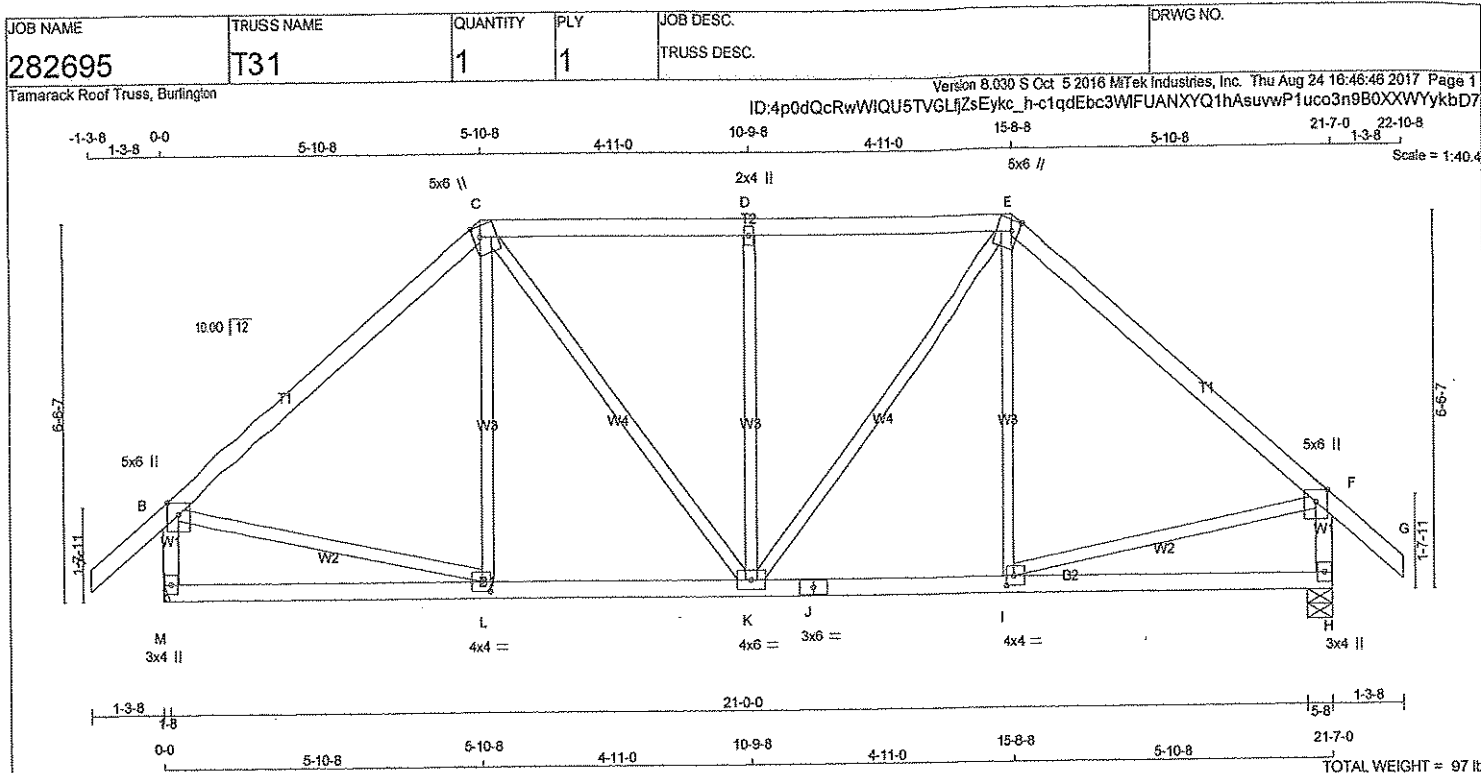
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg

JSI GRIP= 0.90 (O) (INPUT = 0.90)
JSI METAL= 0.42 (M) (INPUT = 1.00)



DWG NO. TAM 42885-17
STRUCTURAL
COMPONENT ONLY



LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMW+p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMWV-t	MT20	4.0	4.0	2.00	1.75
J	BS-t	MT20	3.0	6.0		
K	BMWVW-t	MT20	4.0	6.0		
L	BMWVW-t	MT20	4.0	4.0	2.00	1.75
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REORD
JT	GROSS REACTION	GROSS REACTION	DOWN	BRG	BRG
M	1328 0	1328 0	0	5-8	5-8
H	1328 0	1328 0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN	COMPONENT REACTIONS	WIND	DEAD	SOIL
M	1074	623/0	227/0	0/0	224/0	0/0
H	1074	623/0	227/0	0/0	224/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.52 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)
FR-TO		FROM TO		FR-TO	
A-B	0/37	-84.3 -84.3	0.12 (1)	L-C	-4/219
B-C	-1107/0	-84.3 -84.3	0.41 (1)	C-K	0/388
C-D	-1089/0	-84.3 -84.3	0.27 (1)	K-D	-503/0
D-E	-1089/0	-84.3 -84.3	0.27 (1)	K-E	0/388
E-F	-1107/0	-84.3 -84.3	0.41 (1)	I-E	-4/219
F-G	0/37	-84.3 -84.3	0.12 (1)	B-L	0/869
M-B	-1260/0	0.0 0.0	0.13 (1)	I-F	0/869
H-F	-1260/0	0.0 0.0	0.13 (1)		
M-L	0/0	-28.0 -28.0	0.23 (3)		
L-K	0/849	-28.0 -28.0	0.33 (2)		
K-J	0/849	-28.0 -28.0	0.33 (2)		
J-I	0/849	-28.0 -28.0	0.33 (2)		
I-H	0/0	-28.0 -28.0	0.23 (3)		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/380 (0.72")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.72")
CALCULATED VERT. DEFL.(TL) = L/899 (0.09")

CSI: TC=0.41 (E-F:1), BC=0.33 (I-K:2), WB=0.35 (D-K:1), SSI=0.20 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

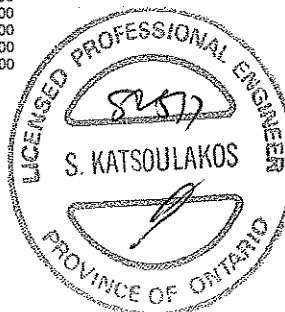
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (L) (INPUT = 0.80)
JSI METAL= 0.32 (L) (INPUT = 1.00)



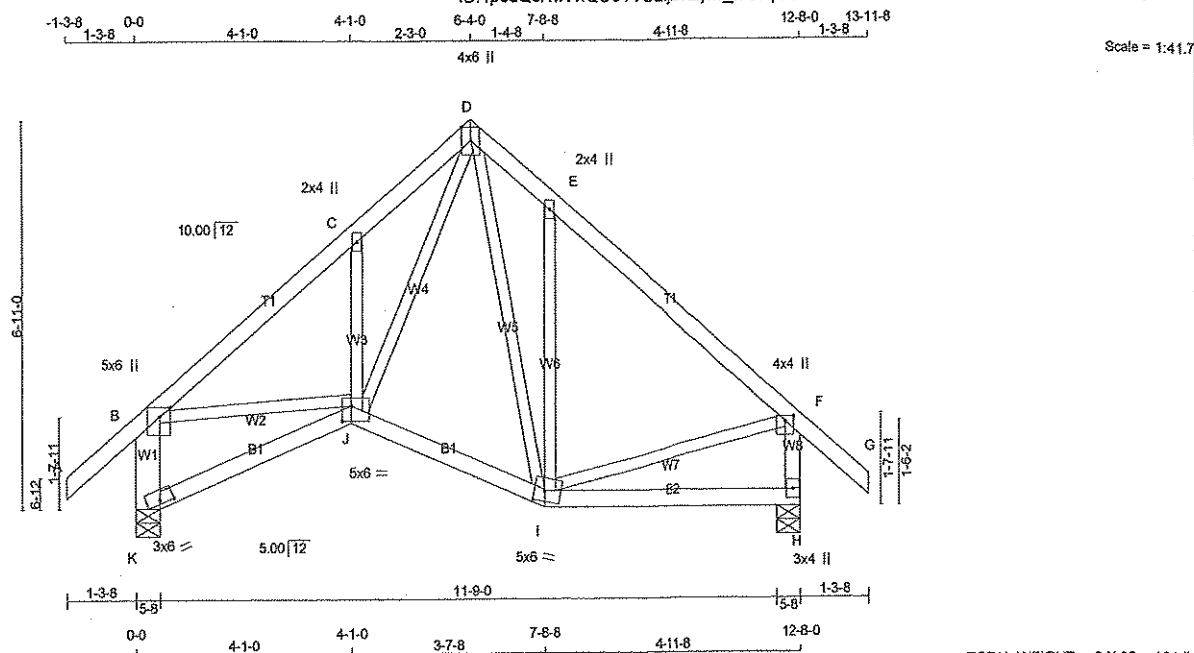
DWG NO. TAM 4209617
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
282695	T32	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 5 Oct 5 2016 MITek Industries, Inc. Thu Aug 24 16:48:46 2017 Page 1

ID:4p0dQcRwWlQU5TVGLfZsEyk_h-c1qdEbc3WlFUANXYQ1hAsuvzH1wao5D9B0XXWYykbD7



TOTAL WEIGHT = 2 X 65 = 131 lb

LUMBER	SIZE	DRY	No.2
N. L. G. A. RULES			
CHORDS	2x4	DRY	No.2
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
K - B	2x6	DRY	No.2
H - F	2x4	DRY	No.2
K - J	2x4	DRY	No.2
J - I	2x4	DRY	No.2
I - H	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TMW+w	MT20	2.0	4.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	4.0	1.00	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BBWWW-m	MT20	5.0	6.0	2.25	2.25
J	BBWWW-p	MT20	5.0	6.0	3.25	2.00
K	BVM1-l	MT20	3.0	6.0	0.75	3.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX	IN-SX
K	828	0	828	0	5-8	5-8	5-8
H	828	0	828	0	5-8	5-8	5-8

UNFACTORED REACTIONS		MAX / MIN. COMPONENT REACTIONS					
JT	1ST LCASE COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
K	663	395 / 0	133 / 0	0 / 0	0 / 0	135 / 0	0 / 0
H	663	395 / 0	133 / 0	0 / 0	0 / 0	135 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO	LENGTH FR-TO
A-B	0 / 37	-84.3 -84.3	10.00
B-C	-830 / 0	-84.3 -84.3	6.25
C-D	-850 / 0	-84.3 -84.3	6.25
D-E	-616 / 0	-84.3 -84.3	6.25
E-F	-544 / 0	-84.3 -84.3	6.25
F-G	0 / 37	-84.3 -84.3	10.00
K-B	-770 / 0	0.0 0.0	7.81
H-F	-758 / 0	0.0 0.0	7.81
K-J	0 / 0	-28.0 -28.0	10.00
J-I	0 / 399	-28.0 -28.0	10.00
I-H	0 / 0	-28.0 -28.0	10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 3.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 46.1 PSF

SPACING = 24.0 IN. GIC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.42")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL) = L/360 (0.42")
CALCULATED VERT. DEFL.(TL) = L/999 (0.07")

CSI: TC=0.22 (E-F:1), BC=0.20 (H-I:2), WB=0.20 (E-I:1), SSI=0.14 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES		PLATE GRIP(DRY) SHEAR		SECTION	
(PSI)	(PLI)	(PSI)	(PLI)	(PSI)	(PLI)
MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1687	822	2284

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (I) (INPUT = 0.90)
JSI METAL= 0.20 (F) (INPUT = 1.00)



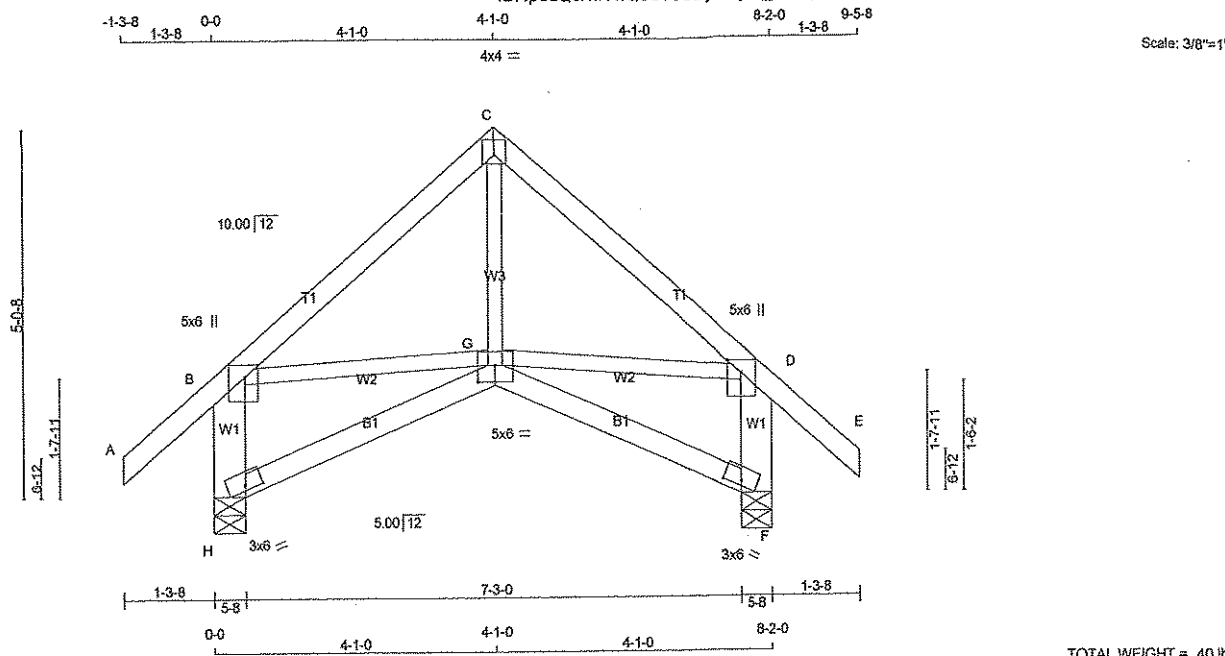
DRWG NO. TAM 4180217
STRUCTURAL
COMPONENT ONLY

JOB NAME 282695	TRUSS NAME T33	QUANTITY 1	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
---------------------------	--------------------------	----------------------	-----------------	--------------------------	----------

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 Mitek Industries, Inc. Thu Aug 24 16:46:46 2017 Page 1

ID:4p0dQcRwWlQU5TVGLfZsEyk_c-h-c1qdEbc3WMFUANXYQ1hAsuv_v1xao7E9B0XXWYykbD7



TOTAL WEIGHT = 40 lb [M/F]

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
H - B	2x6	DRY	No.2
F - D	2x6	DRY	No.2
H - G	2x4	DRY	No.2
G - F	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
DRY: SEASONED LUMBER			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	5.0	6.0	2.00 2.25
C	TTW-p	MT20	4.0	4.0	1.50 2.00
D	TMVW+p	MT20	5.0	6.0	2.00 2.25
F	BVM1-I	MT20	3.0	6.0	0.75 3.00
G	BBWWW-p	MT20	5.0	6.0	2.75 3.00
H	BVM1-I	MT20	3.0	6.0	0.75 3.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG
H	575	0	575	0	5-8
F	575	0	575	0	5-8

UNFACTORED REACTIONS		1ST LCASE	MAX/MIN COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
H	456	280 / 0	86 / 0
F	456	280 / 0	86 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE	VERT. LOAD	LC1 MAX	MEMB.	FORCE
FR-TO				FR-TO	
A-B	0 / 37	-84.3	-84.3 0.12 (1)	G-C	0 / 223
B-C	-382 / 0	-84.3	-84.3 0.18 (1)	B-G	0 / 294
C-D	-382 / 0	-84.3	-84.3 0.18 (1)	G-D	0 / 294
D-E	0 / 37	-84.3	-84.3 0.12 (1)		
H-B	-518 / 0	0.0	0.0 0.04 (1)		
F-D	-518 / 0	0.0	0.0 0.04 (1)		
H-G	0 / 0	-28.0	-28.0 0.14 (3)		
G-F	0 / 0	-28.0	-28.0 0.14 (3)		

DESIGN CRITERIA

SPECIFIED LOADS:		
TOP CH.	LL	= 25.6 PSF
	DL	= 3.0 PSF
BOT CH.	LL	= 10.5 PSF
	DL	= 7.0 PSF
TOTAL LOAD = 46.1 PSF		

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) =	L/360 (0.27")
CALCULATED VERT. DEFL.(LL) =	L/999 (0.02")
ALLOWABLE DEFL.(TL) =	L/360 (0.27")
CALCULATED VERT. DEFL.(TL) =	L/999 (0.04")

CSI: TC=0.18 (C-D:1), BC=0.14 (F-G:3), WB=0.07 (B-G:1), SSI=0.10 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (C) (INPUT = 0.90)
JSI METAL= 0.10 (D) (INPUT = 1.00)



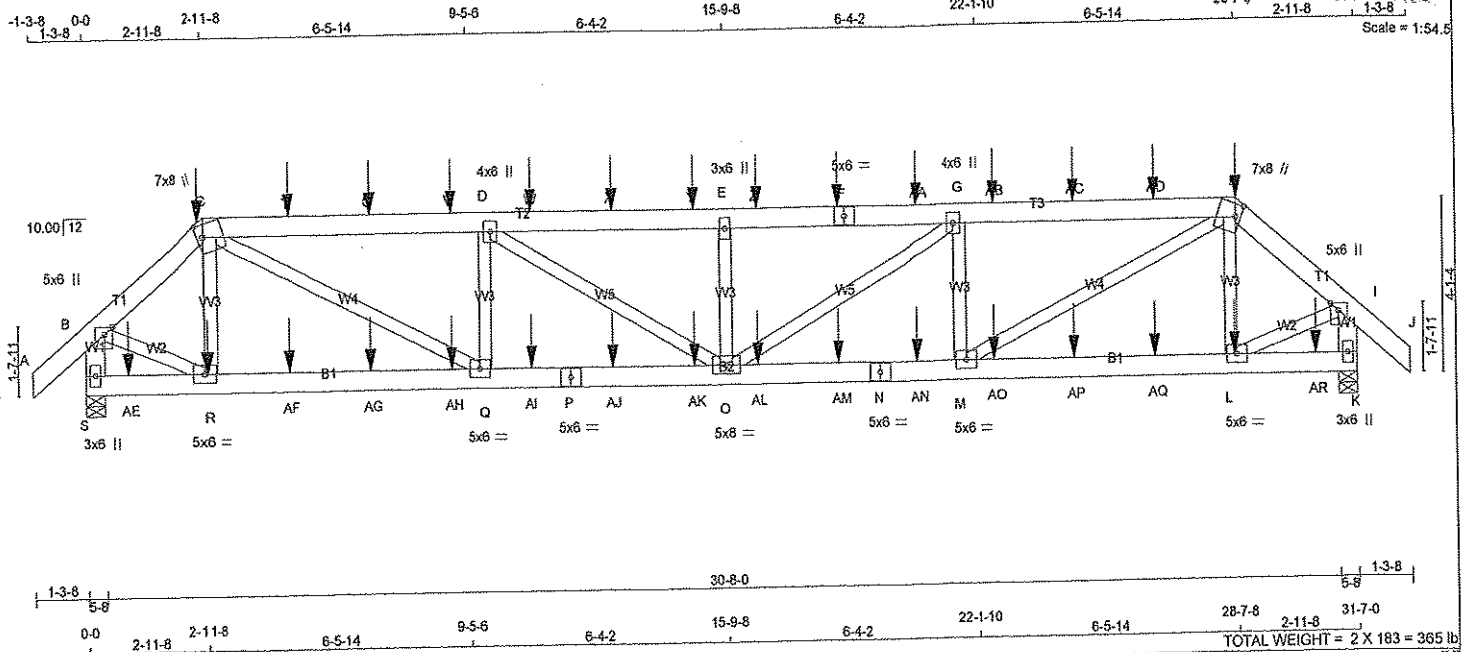
DWG NO. TAM 42008817
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
282695	T100	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 Mitek Industries, Inc. Thu Aug 24 16:46:47 2017 Page 1

ID: P_zJJoeh6kqSygNINRH3eykdWA-4EO?RxdIH2NLnX6kzkCPO5S7?RDFXWJQgG43?ykbD6



LUMBER

N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x6 DRY	No.2	SPF
C - F	2x6 DRY	No.2	SPF
F - H	2x6 DRY	No.2	SPF
H - J	2x6 DRY	No.2	SPF
S - B	2x6 DRY	No.2	SPF
K - I	2x6 DRY	No.2	SPF
S - P	2x6 DRY	No.2	SPF
P - N	2x6 DRY	No.2	SPF
N - K	2x6 DRY	No.2	SPF
ALL WEBS EXCEPT	2x4 DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	2 12	SIDE(122.0)
C - F	2 12	SIDE(61.0)
F - H	2 12	SIDE(61.0)
H - J	2 12	SIDE(122.0)
S - B	2 12	TOP
K - I	2 12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
S - P	2 12	SIDE(183.1)
P - N	2 12	SIDE(0.0)
N - K	2 12	SIDE(183.1)

WEBS : (0.122"x3") SPIRAL NAILS
2x4 1 6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3/4 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN MUST BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWV+p	MT20	5.0	6.0	2.00	2.25
C	TTWV+m	MT20	7.0	8.0	3.50	1.50
D	TMWV+t	MT20	4.0	6.0		
E	TMWV+w	MT20	3.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMWV+t	MT20	4.0	6.0		
H	TTWV+m	MT20	7.0	8.0	3.50	1.50
I	TMWV+p	MT20	5.0	6.0	2.00	2.25
K	BMV1+p	MT20	3.0	6.0		
L, M, Q, R						
L	BMWV-t	MT20	5.0	6.0		
N	BS-t	MT20	5.0	6.0		
O	BMWV-t	MT20	5.0	6.0		
P	BS-t	MT20	5.0	6.0		
S	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG IN-SX	REQRD BRG IN-SX
JT	VERT	HORZ	UPLIFT	
S	3137	0	3137	0
K	3109	0	3109	0

UNFACTORED REACTIONS

	1ST LCASE	MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	2564	1442 / 0	571 / 0	0 / 0	0 / 0	551 / 0	0 / 0
K	2545	1423 / 0	571 / 0	0 / 0	0 / 0	551 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.43 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. FACTORED HORIZ. LOAD LC1 (PLF)	MAX. FACTORED UNBRACED LENGTH (FT)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD LC1 (PLF)
FR-TO					FR-TO		
A-B	0 / 39	-84.3	-84.3	0.03 (1)	10.00	R-C	-535 / 69
B-C	-3136 / 0	-84.3	-84.3	0.05 (1)	6.20	C-Q	0 / 3776
C-T	-5721 / 0	-84.3	-84.3	0.28 (1)	4.69	Q-D	-1463 / 0
T-U	-5721 / 0	-84.3	-84.3	0.28 (1)	4.69	D-O	0 / 946
U-V	-5721 / 0	-84.3	-84.3	0.28 (1)	4.69	O-E	-707 / 0
V-D	-5721 / 0	-84.3	-84.3	0.28 (1)	4.69	E-G	0 / 954
D-W	-6526 / 0	-84.3	-84.3	0.29 (1)	4.43	M-G	-1471 / 0
W-X	-6526 / 0	-84.3	-84.3	0.29 (1)	4.43	M-H	0 / 3793
X-Y	-6526 / 0	-84.3	-84.3	0.29 (1)	4.43	L-H	-527 / 69
Y-E	-6526 / 0	-84.3	-84.3	0.29 (1)	4.43	B-R	0 / 2545
E-Z	-6526 / 0	-84.3	-84.3	0.29 (1)	4.43	L-I	0 / 2521
Z-F	-6526 / 0	-84.3	-84.3	0.29 (1)	4.43		
F-AA	-6526 / 0	-84.3	-84.3	0.29 (1)	4.43		
AA-G	-6526 / 0	-84.3	-84.3	0.29 (1)	4.43		
G-AB	-5713 / 0	-84.3	-84.3	0.28 (1)	4.69		
AB-AC	-5713 / 0	-84.3	-84.3	0.28 (1)	4.69		
AC-AD	-5713 / 0	-84.3	-84.3	0.28 (1)	4.69		
AD-H	-5713 / 0	-84.3	-84.3	0.28 (1)	4.69		
H-I	-3108 / 0	-84.3	-84.3	0.05 (1)	6.21		
I-J	0 / 39	-84.3	-84.3	0.03 (1)	10.00		
S-B	-3130 / 0	0.0	0.0	0.11 (1)	7.80		
K-I	-3102 / 0	0.0	0.0	0.11 (1)	7.80		

S-AE	0 / 0	-28.0	-28.0	0.10 (2)	10.00
AE-R	0 / 0	-28.0	-28.0	0.10 (2)	10.00
R-AF	0 / 2374	-28.0	-28.0	0.23 (1)	10.00
AF-AG	0 / 2374	-28.0	-28.0	0.23 (1)	10.00
AG-AH	0 / 2374	-28.0	-28.0	0.23 (1)	10.00
AH-Q	0 / 2374	-28.0	-28.0	0.23 (1)	10.00
Q-AI	0 / 5721	-28.0	-28.0	0.43 (1)	10.00
AI-P	0 / 5721	-28.0	-28.0	0.43 (1)	10.00
P-AJ	0 / 5721	-28.0	-28.0	0.43 (1)	10.00
AJ-AK	0 / 5721	-28.0	-28.0	0.43 (1)	10.00
AK-O	0 / 5721	-28.0	-28.0	0.43 (1)	10.00
O-AL	0 / 5714	-28.0	-28.0	0.43 (1)	10.00
AL-AM	0 / 5714	-28.0	-28.0	0.43 (1)	10.00
AM-N	0 / 5714	-28.0	-28.0	0.43 (1)	10.00
N-AN	0 / 5714	-28.0	-28.0	0.43 (1)	10.00
AN-M	0 / 5714	-28.0	-28.0	0.43 (1)	10.00
M-AO	0 / 2351	-28.0	-28.0	0.22 (1)	10.00

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
	DL = 3.0 PSF
BOT CH.	LL = 10.5 PSF
	DL = 7.0 PSF
TOTAL LOAD	= 46.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***

ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.05")
CALCULATED VERT. DEFL.(LL)= L/999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (1.05")
CALCULATED VERT. DEFL.(TL)= L/999 (0.25")

CSI: TC=0.29 (E-G:1), BC=0.43 (O-Q:1),
WB=0.34 (H-M:1), SSI=0.18 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (C) (INPUT = 0.90)
JSI METAL= 0.48 (N) (INPUT = 1.00)



DRWG NO. TAM 4009
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
282695	T100	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Thu Aug 24 16:46:47 2017 Page 2

ID: P_zlJoeh6kqSygNINRH3eykdwA-4EO?RxdIH2NLnX6kzkCPO5S7?RDFXWQJQgG43?ykbD6

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 319.1 lbs FACTORED DOWN AT 2-11-8, 283.2 lbs FACTORED DOWN AT 28-7-8, 101.4 lbs FACTORED DOWN AT 5-0-4, 101.4 lbs FACTORED DOWN AT 7-0-4, 101.4 lbs FACTORED DOWN AT 9-0-4, 101.4 lbs FACTORED DOWN AT 11-0-4, 101.4 lbs FACTORED DOWN AT 13-0-4, 101.4 lbs FACTORED DOWN AT 15-0-4, 101.4 lbs FACTORED DOWN AT 16-6-12, 101.4 lbs FACTORED DOWN AT 18-6-12, 101.4 lbs FACTORED DOWN AT 20-6-12, 101.4 lbs FACTORED DOWN AT 22-6-12, AND 101.4 lbs FACTORED DOWN AT 24-6-12, AND 101.4 lbs FACTORED DOWN AT 26-6-12 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 1-0-4, 69.9 lbs FACTORED DOWN AT 3-0-4, 69.9 lbs FACTORED DOWN AT 5-0-4, 69.9 lbs FACTORED DOWN AT 7-0-4, 69.9 lbs FACTORED DOWN AT 9-0-4, 69.9 lbs FACTORED DOWN AT 11-0-4, 69.9 lbs FACTORED DOWN AT 13-0-4, 69.9 lbs FACTORED DOWN AT 15-0-4, 69.9 lbs FACTORED DOWN AT 16-6-12, 69.9 lbs FACTORED DOWN AT 18-6-12, 69.9 lbs FACTORED DOWN AT 20-6-12, 69.9 lbs FACTORED DOWN AT 22-6-12, 69.9 lbs FACTORED DOWN AT 24-6-12, 69.9 lbs FACTORED DOWN AT 26-6-12, AND 69.9 lbs FACTORED DOWN AT 28-6-12, AND 69.9 lbs FACTORED DOWN AT 30-6-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

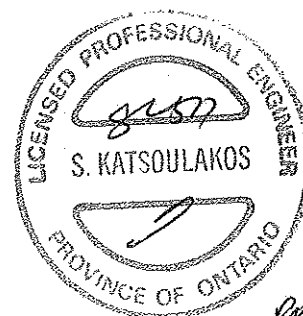
LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM	TO		FR-TO		
AO-AP	0 / 2351	-28.0	-28.0	0.22 (1)	10.00		
AP-AO	0 / 2351	-28.0	-28.0	0.22 (1)	10.00		
AQ-L	0 / 2351	-28.0	-28.0	0.22 (1)	10.00		
L-AR	0 / 0	-28.0	-28.0	0.10 (2)	10.00		
AR-K	0 / 0	-28.0	-28.0	0.10 (2)	10.00		

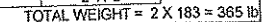
FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-11-8	-16	-18	---	FRONT	VERT	DEAD
C	2-11-8	-141	-141	---	FRONT	VERT	TOTAL
C	2-11-8	-162	-162	---	FRONT	VERT	SNOW
F	18-6-12	-101	-101	---	FRONT	VERT	TOTAL
H	28-7-8	-16	-18	---	FRONT	VERT	DEAD
H	28-7-8	-105	-105	---	FRONT	VERT	TOTAL
H	28-7-8	-162	-162	---	FRONT	VERT	SNOW
L	28-6-12	-40	-70	---	FRONT	VERT	TOTAL
R	3-0-4	-40	-70	---	FRONT	VERT	TOTAL
T	5-0-4	-101	-101	---	FRONT	VERT	TOTAL
U	7-0-4	-101	-101	---	FRONT	VERT	TOTAL
V	9-0-4	-101	-101	---	FRONT	VERT	TOTAL
W	11-0-4	-101	-101	---	FRONT	VERT	TOTAL
X	13-0-4	-101	-101	---	FRONT	VERT	TOTAL
Y	15-0-4	-101	-101	---	FRONT	VERT	TOTAL
Z	16-6-12	-101	-101	---	FRONT	VERT	TOTAL
AA	20-6-12	-101	-101	---	FRONT	VERT	TOTAL
AB	22-6-12	-101	-101	---	FRONT	VERT	TOTAL
AC	24-6-12	-101	-101	---	FRONT	VERT	TOTAL
AD	26-6-12	-101	-101	---	FRONT	VERT	TOTAL
AE	1-0-4	-40	-70	---	FRONT	VERT	TOTAL
AF	5-0-4	-40	-70	---	FRONT	VERT	TOTAL
AG	7-0-4	-40	-70	---	FRONT	VERT	TOTAL
AH	9-0-4	-40	-70	---	FRONT	VERT	TOTAL
AI	11-0-4	-40	-70	---	FRONT	VERT	TOTAL
AJ	13-0-4	-40	-70	---	FRONT	VERT	TOTAL
AK	15-0-4	-40	-70	---	FRONT	VERT	TOTAL
AL	16-6-12	-40	-70	---	FRONT	VERT	TOTAL
AM	18-6-12	-40	-70	---	FRONT	VERT	TOTAL
AN	20-6-12	-40	-70	---	FRONT	VERT	TOTAL
AO	22-6-12	-40	-70	---	FRONT	VERT	TOTAL
AP	24-6-12	-40	-70	---	FRONT	VERT	TOTAL
AQ	26-6-12	-40	-70	---	FRONT	VERT	TOTAL
AR	30-6-12	-40	-70	---	FRONT	VERT	TOTAL



DWG NO. TAM 42809-17
STRUCTURAL
COMPONENT ONLY

Version 8.030 S Oct 5 2016 MiTek Industries, Inc. Thu Aug 24 17:16:55 2017 Page 1



DESCR
SPF
SPF
SPF
SPF
SPF
SPF
SPF
SPF

CONTINUED ON PAGE 2

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
R	BMWV-t	MT20	5.0	6.0		
S	BMV1+p	MT20	3.0	6.0		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
Y	7-0-4	-40	-70	—	BACK	VERT	TOTAL
Z	9-0-4	-40	-70	—	BACK	VERT	TOTAL

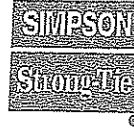
HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 178.3 lbs FACTORED DOWN AT 2-11-8, 140.9 lbs FACTORED DOWN AT 2-11-8, 101.4 lbs FACTORED DOWN AT 5-0-4, AND 101.4 lbs FACTORED DOWN AT 7-0-4, AND 101.4 lbs FACTORED DOWN AT 9-0-4 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 1-0-4, 69.9 lbs FACTORED DOWN AT 3-0-4, 69.9 lbs FACTORED DOWN AT 5-0-4, 69.9 lbs FACTORED DOWN AT 7-0-4, AND 69.9 lbs FACTORED DOWN AT 9-0-4, AND 2158.5 lbs FACTORED DOWN AT 8-10-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.



DWG NO. TAM 4207017
STRUCTURAL
COMPONENT ONLY

LUS - Double Shear Joist Hangers



All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

MATERIAL: 18 gauge

FINISH: G90 galvanized

DESIGN:

- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

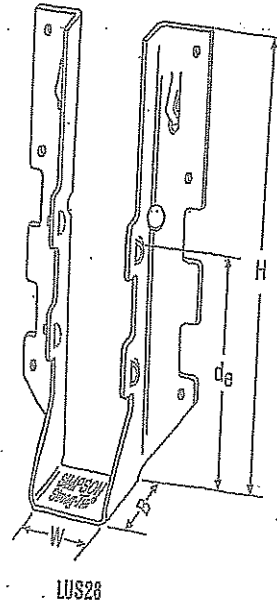
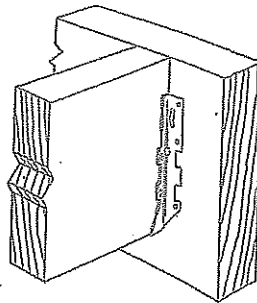
INSTALLATION:

- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/4" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

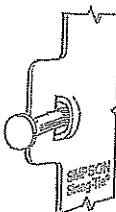
- These hangers cannot be modified.

Typical LUS Installation



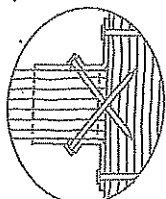
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
		W	H	B	dg1	Face	Joist	D-Fir-L		S-P-F	
								Uplift (K _u =1.15)	Normal (K _n =1.00)	Uplift (K _u =1.15)	Normal (K _n =1.00)
LUS24	18	1 1/8	3 1/4	1 3/4	1 1/8	4-10d	2-10d	710	1630	645	1155
LUS24-2	18	3 1/4	3 1/4	2	1 1/8	4-16d	2-16d	835	2020	590	1435
LUS26	18	1 1/8	4 3/4	1 3/4	3/4	4-10d	4-10d	1420	2170	1290	1630
LUS26-2	18	3 1/4	4 3/4	2	4	4-16d	4-16d	1720	2595	1545	1920
LUS26-3	18	4 3/4	4 3/4	2	3/4	4-16d	4-16d	1720	2595	1545	2340
LUS28	18	1 1/8	6 3/4	1 3/4	3/4	6-10d	4-10d	1420	2520	1290	1790
LUS28-2	18	3 1/4	7	2	4	6-16d	4-16d	1720	3325	1545	2575
LUS28-3	18	4 3/4	6 1/4	2	3/4	6-16d	4-16d	1720	3325	1545	2375
LUS210	18	1 1/8	7 1/8	1 3/4	3/4	8-10d	4-10d	1420	2785	1290	2210
LUS210-2	18	3 1/4	9	2	6	8-16d	6-16d	2580	4500	2320	3195
LUS210-3	18	4 3/4	8 3/4	2	5 1/4	8-16d	6-16d	2580	3345	2320	2375

1. dg is the distance from the seat of the hanger to the highest joist nail.

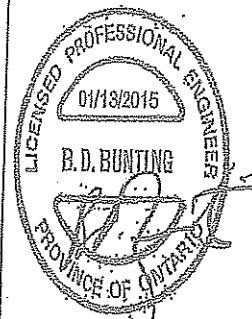


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent
5,603,589



Double Shear Nailing Top View.



LIMIT
STATES
DESIGN

This technical bulletin is effective as of January 1, 2016, and reflects information available as of January 1, 2016. This information is updated periodically and should not be relied upon after December 31, 2016. Contact Simpson Strong-Tie for current information and limited warranty or see www.strongtie.com.

© 2016 Simpson Strong-Tie Company Inc.

T-SPEC LUS 16 1/15 Rev. 12/16

800-999-5099

www.strongtie.com

HUS/LJS - Double Shear Joist Hangers



All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: See table

FINISH: G90 galvanized

DESIGN:

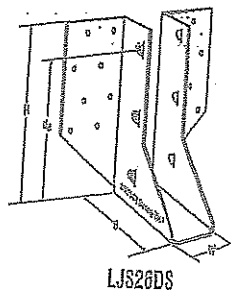
- Factored resistances are in accordance with CSA 086-14
- Uplift resistances have been increased 15%. No further increase is permitted
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

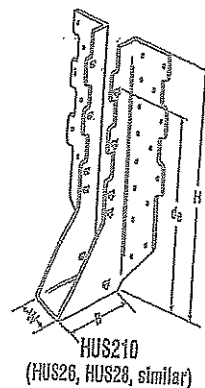
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

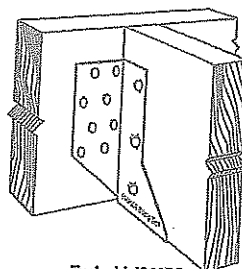
- See current catalogue for options



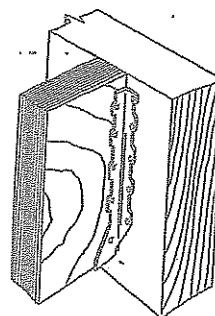
LJS26DS



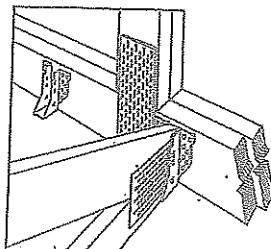
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



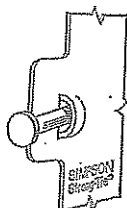
Typical HUS
Installation



Typical HUS Installation
(Truss Designer to provide fastener
quantity for connecting multiple
members together)

Model No.	Ga	Dimensions (in)			Fasteners		Factored Resistance (lbs)				
		W	H	B	d ₀ ¹	Face	Joist	D-Fir-L		S-P-F	
								Uplift (K ₀ =1.15)	Normal (K ₀ =1.00)	Uplift (K ₀ =1.15)	Normal (K ₀ =1.00)
LJS26DS	18	1 ¹ / ₁₆	5	3 ¹ / ₂	4%	16-16d	6-16d	2055	4265	1460	4115
HUS26	18	1%	5%	3	3 ¹ / ₁₆	14-16d	6-16d	2705	4940	2065	3875
HUS28	18	1%	7 ¹ / ₂	3	6 ¹ / ₂	22-16d	8-16d	3605	5365	2875	4345
HUS210	18	1%	9 ¹ / ₂	3	7 ³ / ₂	30-16d	10-16d	4505	5795	4010	4740
HUS1.81/10	18	1 ¹ / ₁₆	9	3	8	30-16d	10-16d	4505	6450	4010	5200

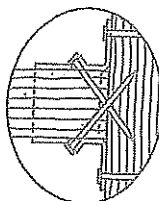
1. d₀ is the distance from the seat of the hanger to the highest joist nail.



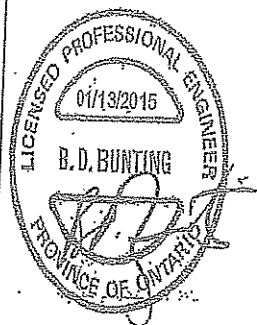
Double Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).
U.S. Patent
5,603,580



Double
Shear
Nailing
Side
View. Do
not bend
tab back.



Double
Shear
Nailing
Top View.



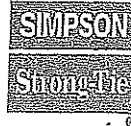
UNITED STATES DESIGN

This technical bulletin is subject to change without notice. It is not intended to replace the manufacturer's instructions. Information is provided for general informational purposes only and should not be relied upon for specific design. Contact Simpson Strong-Tie for more information and limitations. © 2015 Simpson Strong-Tie Company, Inc.

800-999-5099
www.strongtie.com

13PCHUS15-1/16-K12/15

HGUS – Double Shear Joist Hangers



All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: 12 gauge

FINISH: G90 galvanized

DESIGN:

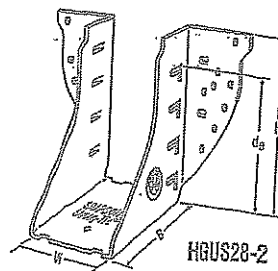
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%.
No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

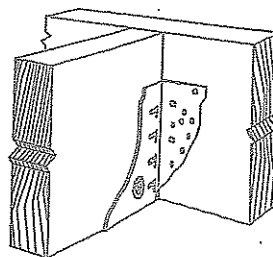
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

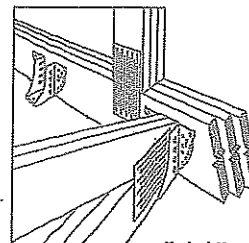
- See current catalogue for options.



HGUS28-2



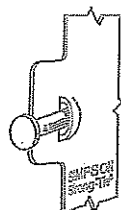
Typical HGUS Installation



Typical HGUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

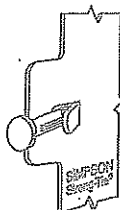
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
		W	H	B	d _p ¹	Face	Joist	D-Fir-L		S-P-F	
								Uplift (K _u =1.15)	Normal (K _n =1.00)	Uplift (K _u =1.15)	Normal (K _n =1.00)
HGUS28	12	1 1/4	5 3/4	5	4 1/4	20-16d	8-16d	2685	6625	2685	5700
HGUS28-2	12	3 3/8	6 7/8	4	4 1/4	20-16d	8-16d	4385	8950	3100	6355
HGUS28-3	12	4 1/8	6 1/2	4	4 1/4	20-16d	8-16d	4385	8950	3100	6355
HGUS28-4	12	6 1/8	6 1/8	4	4 1/4	20-16d	8-16d	4385	8950	3100	6355
HGUS28	12	1 1/4	7 1/4	5	6 1/4	36-16d	12-16d	3310	7675	3100	6800
HGUS28-2	12	3 3/8	7 7/8	4	6 1/4	36-16d	12-16d	6070	12980	4310	9215
HGUS28-3	12	4 1/8	7 1/4	4	6 1/4	36-16d	12-16d	6070	12980	4310	9215
HGUS28-4	12	6 1/8	7 1/8	4	6 1/4	36-16d	12-16d	6070	12980	4310	9215
HGUS210-2	12	3 3/8	9 3/8	4	8 1/4	46-16d	16-16d	6840	14645	4855	10400
HGUS210-3	12	4 1/8	9 1/4	4	8 1/4	46-16d	16-16d	6840	14645	4855	10400
HGUS210-4	12	6 1/8	9 3/8	4	8 1/4	46-16d	16-16d	6840	14645	4855	10400
HGUS212-4	12	6 7/8	10 7/8	4	10 1/4	56-16d	20-16d	7640	14995	5425	10645
HGUS214-4	12	6 1/2	12 1/4	4	11 1/4	68-16d	22-16d	10130	16400	7195	11645

1. d_p is the distance from the seat of the hanger to the highest joist nail.

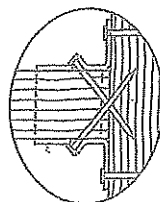


Double Shear Nailing prevents tabs breaking off (available on some models).

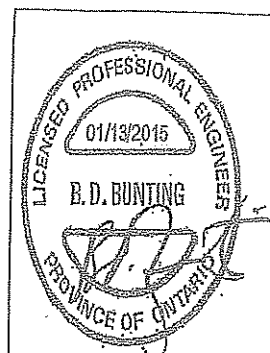
U.S. Patent
5,603,580



Double Shear Nailing Side View. Do not bend tab back.



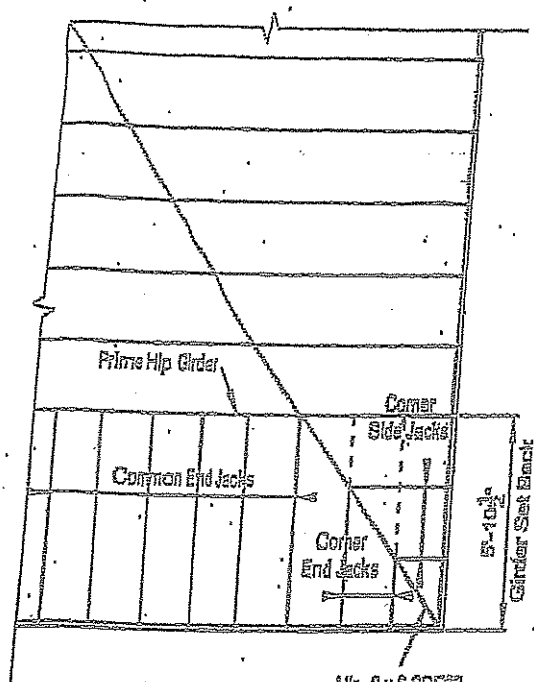
Double Shear Nailing Top View.



DESIGN

1. This technical bulletin is effective until December 31, 2016, and superseded information available until December 31, 2016. The information in this bulletin should not be relied upon after December 31, 2016. Contact Simpson Strong-Tie for current information and limited warranty at www.strongtie.com.
© 2015 Simpson Strong-Tie Company, Inc. SPEC-HGUS15-01/15 Rev. 12/15

800-499-8099
www.strongtie.com



45° Hip End

LUMBER SPECIFICATION

TOP CHORD : 2x4 SPF#2

BOTTOM CHORD : 2x4 SPF#2

WEBS : 2x3 SPF#2

UNLESS OTHERWISE SHOWN

DESIGN LOAD:

TOP CHORD LIVE LOAD : 39.4 P.S.F.

TOP CHORD DEAD LOAD : 3.0 P.S.F.

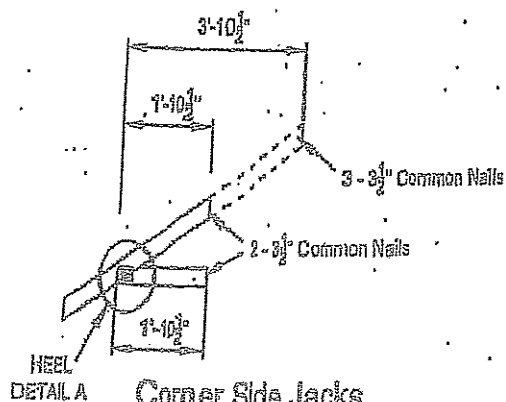
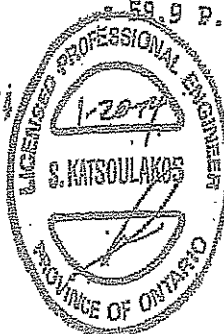
BOTTOM CHORD LIVE LOAD : 10.5 P.S.F.

BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

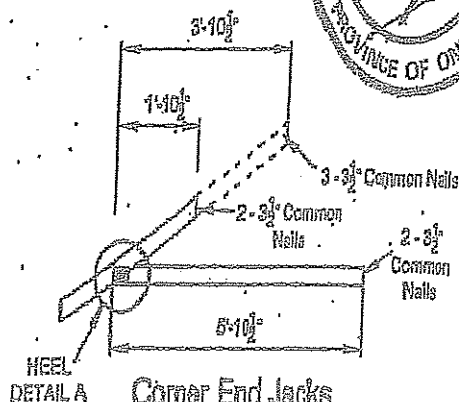
TOTAL LOAD : 59.9 P.S.F.

DWG NO. TIM 2495.1

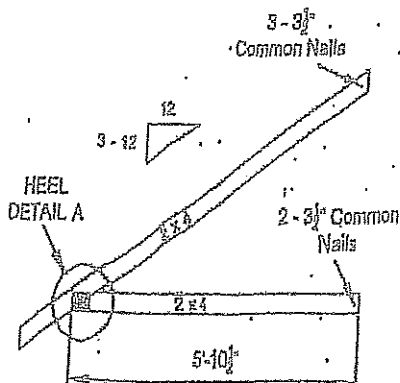
STRUCTURAL
COMPONENT ONLY



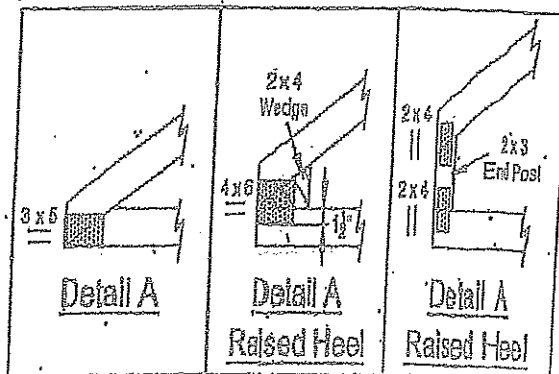
Corner Side Jacks



Corner End Jacks



Common End Jacks



Detail A

Detail A
Raised Heel

Detail A
Raised Heel

NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)

(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, NOL 1M0

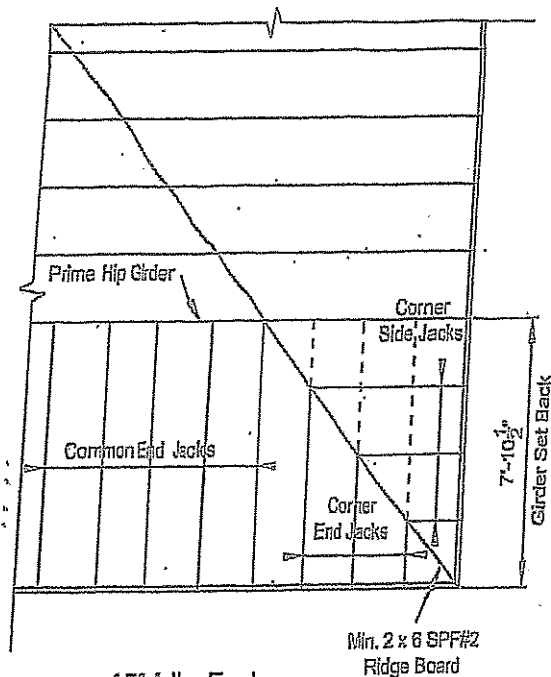
LUMBER SPECIFICATION

TOP CHORD : 2x4 SPF#2
 BOTTOM CHORD : 2x4 SPF#2
 WEBS : 2x3 SPF#2
 UNLESS OTHERWISE SHOWN

DESIGN LOAD:

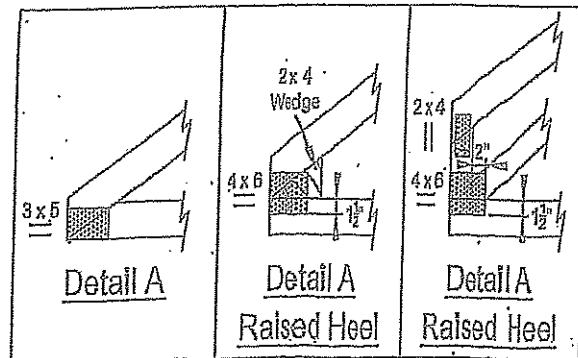
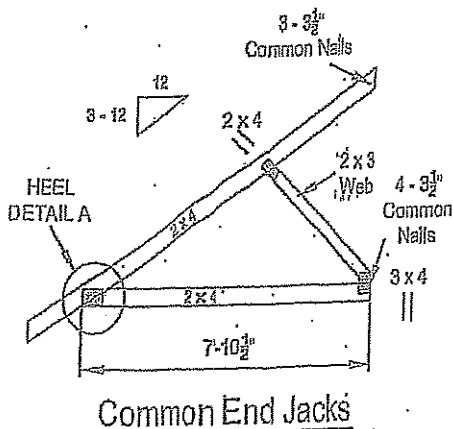
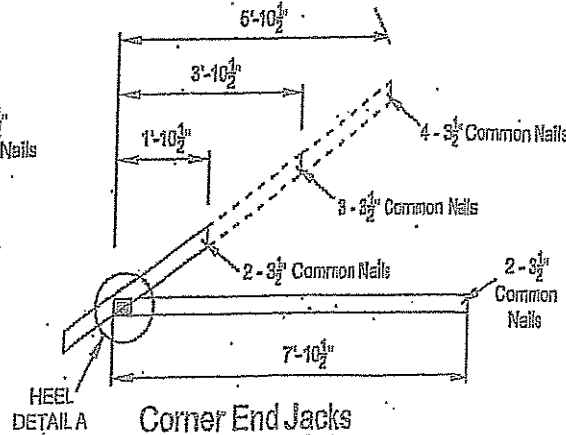
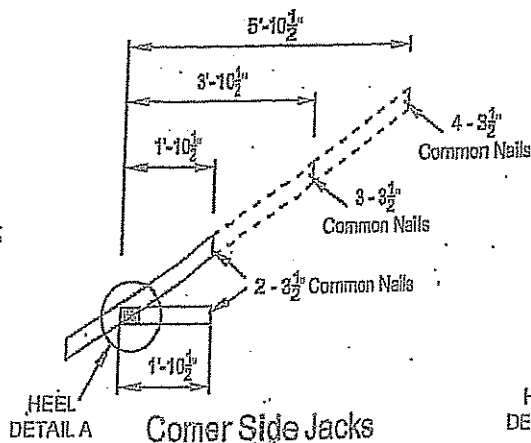
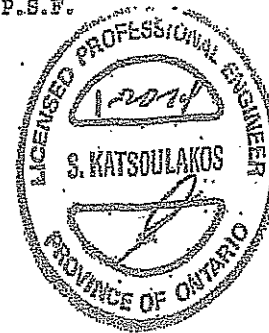
TOP CHORD LIVE LOAD : 34.8 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD : 44.8 P.S.F.



45° Hip End

DWG NO T&M 3503.14
 STRUCTURAL
 COMPONENT ONLY

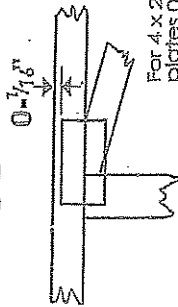


NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
 (TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

Symbols

PLATE LOCATION AND ORIENTATION

Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*Plate location details available in Mitek software or upon request.

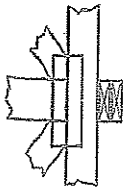
PLATE SIZE

4 x 4
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

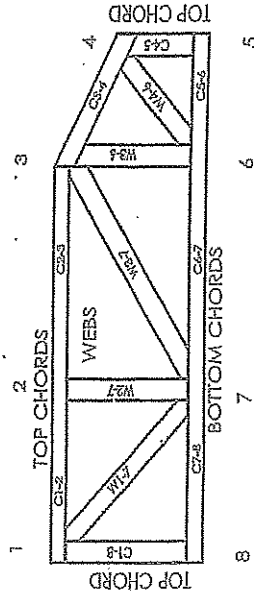


Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

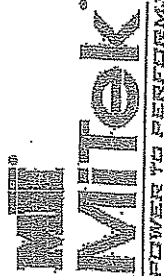
CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

© 2007 Mitek® All Rights Reserved



Mitek Engineering Reference Sheet: MII-7473C rev. 10-08

General Safety Notes

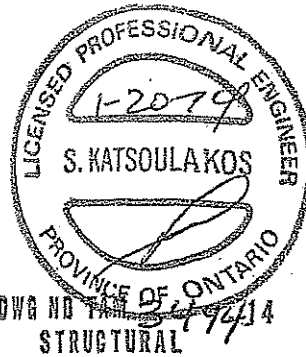
Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system. See BCSI, diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and waste at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

Micro City Engineering Services Inc.
(BCIN: 26064; FIRM BCIN: 29991)

RR #1, Po Box 61
Glencoe, Ontario
N0L 1M0

(519) 287 - 2242; Fax: (519) 287 - 5750 (Call)



Responsibilities:

Micro City Engineering Services is responsible for the design of trusses as individual components.

It is the responsibilities of others to ascertain that the design loads utilized on this (these) drawing(s) meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdiction over such decisions.

All dimensions are to be verified by the owner, contractor, architect, or other authority having input over such decisions prior to truss component manufacture. At no time shall Micro City Engineering Services Inc. or its employees be responsible for dimension errors.

Micro City Engineering Services Inc. bears no responsibility for the erection of any truss components. Persons erecting truss components are cautioned to seek professional advice regarding temporary and permanent bracing systems and to be totally familiar with all aspects of truss erection prior to proceeding on any truss component erection job. Any bracing shown on Micro City Engineering Services Inc. or Tamarack Roof Trusses Inc. sealed or unsealed truss component drawings is specified for the single truss component in question and is identified as an integral part of the design for that particular truss component but is not meant to represent the only required bracing for that particular truss component when installed as a component in a series of truss components in a roof truss system.

~~It is the truss manufacturer's responsibility to ensure that trusses are manufactured in accordance with Micro City Engineering Services Inc. specifications outlined below:~~

SPECIFICATIONS:

Truss components sealed by Micro City Engineering Services Inc. must conform to the relevant sections of the current Building Code of Ontario and Canada (Part 4 or Part 9) or the current Farm Building Code of Canada in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the Truss Plate Institute of Canada (TPIC). All unit lumber and nailing stresses identified on truss component design drawings and/or used in the design of individual truss components shall conform to the current CSA Wood Design standard identified in the current Building Code and TPIC Design Standards.

The lumber used to manufacture any truss component is to conform to the specified size and grade identified on the truss drawing.

The lumber used in the manufacture of any truss component is not to exceed 19% during its service use unless specifically noted on the truss drawing.

The lumber used in the manufacture of any truss component is not to be treated with any chemicals during its service life unless specifically noted on the truss drawing.

Connector plates shall be applied to both faces of the truss component at each joint and shall be positioned exactly as specified.

The top chord of any truss component is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the sealed truss component drawing but not exceeding 24" o/c (Part 9 design) and not exceeding 48" o/c (Part 4 or Agricultural design).

When a truss component is to be installed with no rigid ceiling attached directly to the bottom chord, then the bottom chord is to be laterally braced at intervals not exceeding 3m (or 10'-0").

All sealed or unsealed truss component drawings provided by Micro City Engineering Services Inc. Or Tamarack Roof Trusses Inc. should be read in conjunction with the following:

Warning-Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473C rev 10-'08 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer - not the truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpik.ca and BCSI Building Component Safety Information available from the Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA, 22314.