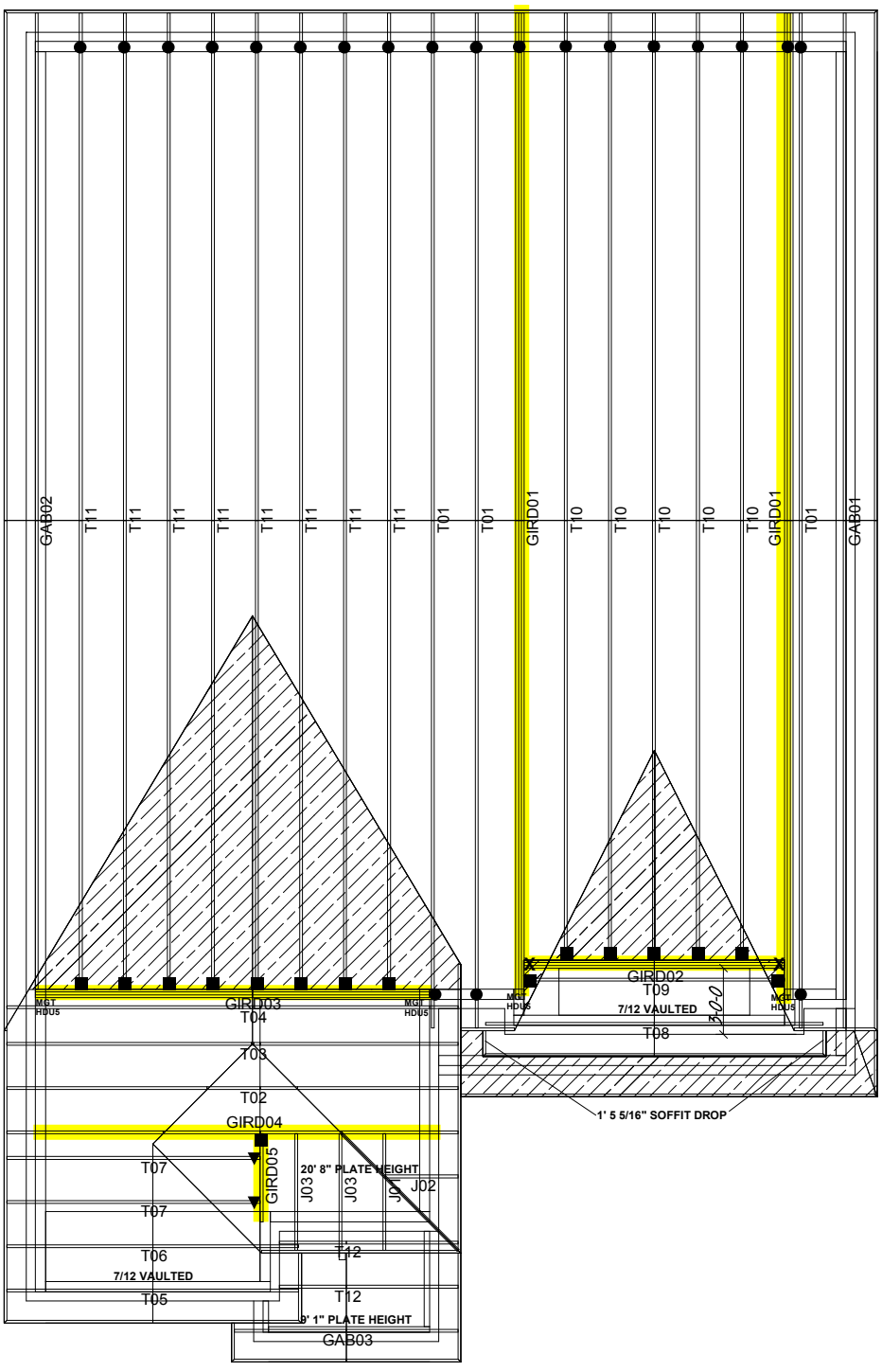
TRUSSES BY KOTT



RECEIVED
TOWN OF MILTON
MAR 29, 2017
JUNIPER 9
BUILDING DIVISION

TOWN OF MILTON
PLANNING AND DEVELOPMENT
JUNIPER 9 MODEL
BUILDING: REVIEWED
SCOTT SHERRIFFS MAR 30, 2017
PLANS EXAMINER DATE
Neither the issuance of a permit nor carrying out of inspections by the Town of Milton relives the owner from full responsibility for compliance with the provisions of the Ontario Building Code Act and the Ontario Building Code, both as amended, as well as other applicable statutes and regulations of the Province on Ontario, By-laws of the Region of Halton and Town of Milton

CONVENTIONAL FRAMING BY OTHERS

ALL CONVENTIONAL FRAMING TO CONFORM WITH PART 9 OF THE O.B.C
ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4 S.P.F. @ 24"O/C WITH A 2x4 VERTICAL POST TO THE TRUSS UNDERNEATH EACH CROSS POINT. VERTICAL POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END POINT AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

HANGER LEGEND:

▼ LUS24 ■ LJS26DS
● (2)H2.5A ✕ THGQ3

SIZE AND LOCATION OF CONVENTIONAL FRAMING IS APPROXIMATE. ALL AREAS MAY NOT BE SHOWN. REFER TO ARCHITECTURAL PLANS FOR DETAILS.

Model: **JUNIPER 9 EL 2**
Customer: GREENPARK
Project: LECCO RIDGE
Location: MILTON
Date: 3/23/2017 Drawn by: BB

ENGINEERING NOTE PAGE (ENP-1)
PLEASE READ PRIOR TO INSTALLATION

RESPONSIBILITIES

THIS DESIGN IS FOR AN INDIVIDUAL BUILDING COMPONENT AND HAS BEEN BASED ON INFORMATION PROVIDED BY THE DESIGN OFFICE OF KOTT LUMBER. THE UNDERSIGNED ENGINEER DISCLAIMS ANY RESPONSIBILITY FOR DAMAGES AS A RESULT OF FAULTY OR INCORRECT INFORMATION, SPECIFICATION AND/OR DESIGNS FURNISHED TO THE ENGINEER. THE UNDERSIGNED ENGINEER IS ONLY RESPONSIBLE FOR THE STRUCTURAL INTEGRITY OF THIS BUILDING COMPONENT FOR THE CONDITIONS AND LOADS SHOWN ON THIS DRAWING. THE STRUCTURAL INTEGRITY OF THE BUILDING AND THE VERIFICATION OF THE DIMENSIONS AND THE DESIGN LOADS USED ARE THE RESPONSIBILITY OF THE BUILDING DESIGNER.

TRUSSES ARE DESIGNED IN CONFORMANCE WITH THE RELEVANT SECTIONS OF THE NATIONAL BUILDING CODE OF CANADA OR THE CANADIAN CODE FOR FARM BUILDINGS, WHICHEVER APPLIES TO THE BUILDING TYPE INDICATED ON THE DRAWING

IT IS THE RESPONSIBILITY OF KOTT LUMBER TO ENSURE THAT TRUSSES ARE MANUFACTURED IN CONFORMANCE WITH THESE DESIGNS AND WITH THE SPECIFICATIONS OUTLINED BELOW. THE UNDERSIGNED ENGINEER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

USE AND OCCUPANCY

- The building is of the type indicated on the drawing

LOADING

- The truss loading intensity and distribution as well as load transfer mechanism is that indicated on the drawing
- No buildings, trees, parapets or other projections higher than the roof for which the trusses are used are located within a distance less than ten (10) times the difference in height, or five metres (16 ft) whichever is greater, unless the drawing indicates that the snow drifting has been taken into account

HANDLING, INSTALLATION AND BRACING

- The trusses must be handled and installed by a qualified professional as per the supplied document titled *Information for Truss Installers* and the BCSI-B1 and BCSI-B3 Summary Sheets
- The compression chords are laterally braced by continuous rigid diaphragm sheathing or as specified on the drawing
- Temporary and permanent bracing must be installed as indicated on the truss drawing and according to the BCSI-B1 and BCSI-B3 Summary Sheets. Bracing for the lateral stability of the truss is to be provided by the building designer
- **It is recommended that a Professional Engineer's advice be obtained for the bracing of trusses spanning more than 12.37m (40'-7")**

SUPPORTS

- The trusses are to be supported at the bearing points indicated and anchored to the supports where considered necessary by the designer of the overall structure
- Bearing sizes shown are the minimum required to prevent crushing of the truss members and do not necessarily take into account stability of the overall building structure
- Elevation of bearings must be carefully checked and shimmed to alignment for solid bearings
- Adequate wood truss bearing is the responsibility of the building designer.

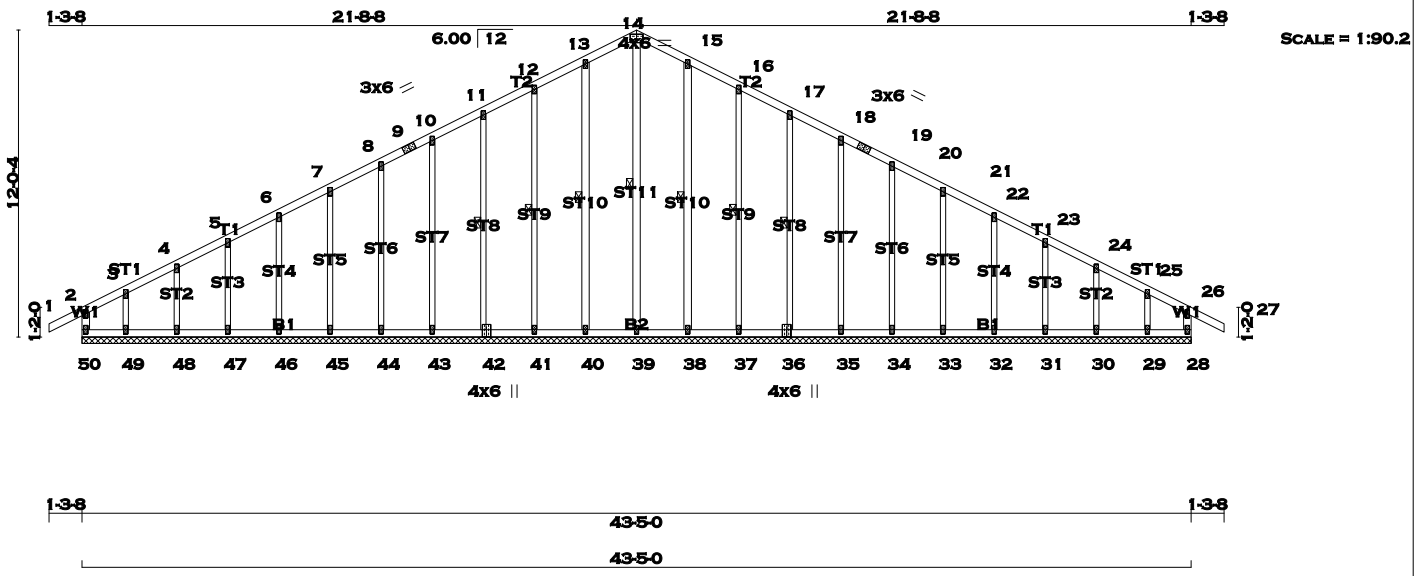
DIMENSIONS

- Geometry of the truss and dimensions indicated on the drawing are identical to those of the installed truss.

WHERE CONTINUOUS LATERAL BRACING IS REQUIRED FOR WEBS BUT CAN NOT BE PROVIDED SUBSTITUTE EACH WITH ONE SPF #2 2" X 4" T-BRACE COVERING 90% OF WEB LENGTH AND FASTENED TO EDGE OF WEB USING 3 1/4" SPIRAL NAILS @ 6" C/C

RECEIVED
TOWN OF MILTON
MAR 29, 2017
JUNIPER 9
BUILDING DIVISION

01/29/2013



TOTAL WEIGHT = 227 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
50 - 2	2x4	DRY	No.2	SPF
1 - 9	2x4	DRY	No.2	SPF
9 - 14	2x4	DRY	No.2	SPF
14 - 19	2x4	DRY	No.2	SPF
19 - 27	2x4	DRY	No.2	SPF
28 - 26	2x4	DRY	No.2	SPF
50 - 42	2x4	DRY	No.2	SPF
42 - 36	2x4	DRY	No.2	SPF
36 - 28	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
39 - 14	2x4	DRY	No.2	SPF
40 - 13	2x4	DRY	No.2	SPF
38 - 15	2x4	DRY	No.2	SPF
ALL GABLE WEBS				
	2x3	DRY	No.2	SPF
EXCEPT				
ST1	2x4	DRY	No.2	SPF
ST2	2x4	DRY	No.2	SPF
ST12	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMV+p	MT20	2.0	4.0		
3, 4, 5, 6, 7, 8, 10, 11, 12, 13, 15, 16, 17, 18, 20, 21, 22, 23, 24, 25						
3	TMW+w	MT20	2.0	4.0		
9	TS-t	MT20	3.0	6.0		
14	TTW-p	MT20	4.0	6.0		
19	TS-t	MT20	3.0	6.0		
26	TMV+p	MT20	2.0	4.0		
28	BMV1+p	MT20	2.0	4.0		
29, 30, 31, 32, 33, 34, 35, 37, 38, 39, 40, 41, 43, 44, 45, 46, 47, 48, 49						
29	BMW1+w	MT20	2.0	4.0		
36	BSW1+l	MT20	4.0	6.0		
42	BSW1+l	MT20	4.0	6.0		
50	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 14-39, 13-40, 12-41, 11-42, 15-38, 16-37, 17-36. DBS = 20-0-0 . CBF = 22 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
50- 2	-194 / 0	0.0	0.0	0.02 (1)	7.81	39-14	-178 / 0	0.14 (1)
1- 2	0 / 23	-77.3	-77.3	0.10 (1)	10.00	40-13	-162 / 0	0.10 (1)
2- 3	-11 / 0	-77.3	-77.3	0.07 (1)	6.25	41-12	-150 / 0	0.10 (1)
3- 4	0 / 8	-77.3	-77.3	0.04 (1)	10.00	42-11	-154 / 0	0.08 (1)
4- 5	0 / 8	-77.3	-77.3	0.04 (1)	10.00	43-10	-154 / 0	0.19 (1)
5- 6	0 / 12	-77.3	-77.3	0.04 (1)	10.00	44- 8	-154 / 0	0.13 (1)
6- 7	0 / 14	-77.3	-77.3	0.04 (1)	10.00	45- 7	-154 / 0	0.09 (1)
7- 8	0 / 16	-77.3	-77.3	0.04 (1)	10.00	46- 6	-154 / 0	0.06 (1)
8- 9	0 / 18	-77.3	-77.3	0.04 (1)	10.00	47- 5	-151 / 0	0.04 (1)
9-10	0 / 18	-77.3	-77.3	0.04 (1)	10.00	48- 4	-162 / 0	0.03 (1)
10-11	0 / 19	-77.3	-77.3	0.04 (1)	10.00	49- 3	-106 / 0	0.02 (1)
11-12	0 / 23	-77.3	-77.3	0.04 (1)	10.00	38-15	-162 / 0	0.10 (1)
12-13	0 / 26	-77.3	-77.3	0.04 (1)	10.00	37-16	-150 / 0	0.10 (1)
13-14	0 / 24	-77.3	-77.3	0.04 (1)	10.00	36-17	-154 / 0	0.08 (1)
14-15	0 / 24	-77.3	-77.3	0.04 (1)	10.00	35-18	-154 / 0	0.19 (1)
15-16	0 / 26	-77.3	-77.3	0.04 (1)	10.00	34-20	-154 / 0	0.13 (1)
16-17	0 / 23	-77.3	-77.3	0.04 (1)	10.00	33-21	-154 / 0	0.09 (1)
17-18	0 / 19	-77.3	-77.3	0.04 (1)	10.00	32-22	-154 / 0	0.06 (1)
18-19	0 / 18	-77.3	-77.3	0.04 (1)	10.00	31-23	-151 / 0	0.04 (1)
19-20	0 / 18	-77.3	-77.3	0.04 (1)	10.00	30-24	-162 / 0	0.03 (1)
20-21	0 / 16	-77.3	-77.3	0.04 (1)	10.00	29-25	-106 / 0	0.02 (1)
21-22	0 / 14	-77.3	-77.3	0.04 (1)	10.00			
22-23	0 / 12	-77.3	-77.3	0.04 (1)	10.00			
23-24	0 / 8	-77.3	-77.3	0.04 (1)	10.00			
24-25	0 / 8	-77.3	-77.3	0.04 (1)	10.00			
25-26	-11 / 0	-77.3	-77.3	0.07 (1)	6.25			
26-27	0 / 23	-77.3	-77.3	0.10 (1)	10.00			
28-26	-194 / 0	0.0	0.0	0.02 (1)	7.81			
50-49	0 / 1	-17.5	-17.5	0.02 (1)	10.00			
49-48	-4 / 0	-17.5	-17.5	0.01 (4)	10.00			
48-47	-8 / 0	-17.5	-17.5	0.01 (4)	10.00			
47-46	-11 / 0	-17.5	-17.5	0.01 (4)	6.25			
46-45	-13 / 0	-17.5	-17.5	0.01 (4)	6.25			
45-44	-15 / 0	-17.5	-17.5	0.01 (4)	6.25			
44-43	-16 / 0	-17.5	-17.5	0.02 (4)	6.25			
43-42	-17 / 0	-17.5	-17.5	0.02 (4)	6.25			
42-41	-21 / 0	-17.5	-17.5	0.01 (4)	6.25			
41-40	-22 / 0	-17.5	-17.5	0.01 (4)	6.25			
40-39	-23 / 0	-17.5	-17.5	0.01 (4)	6.25			
39-38	-23 / 0	-17.5	-17.5	0.01 (4)	6.25			
38-37	-22 / 0	-17.5	-17.5	0.01 (4)	6.25			
37-36	-21 / 0	-17.5	-17.5	0.01 (4)	6.25			
36-35	-17 / 0	-17.5	-17.5	0.02 (4)	6.25			
35-34	-16 / 0	-17.5	-17.5	0.02 (4)	6.25			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.10 (1-2:1) , BC=0.02 (49-50:1) , WB=0.19 (10-43:1) , SSI=0.07 (1-2:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

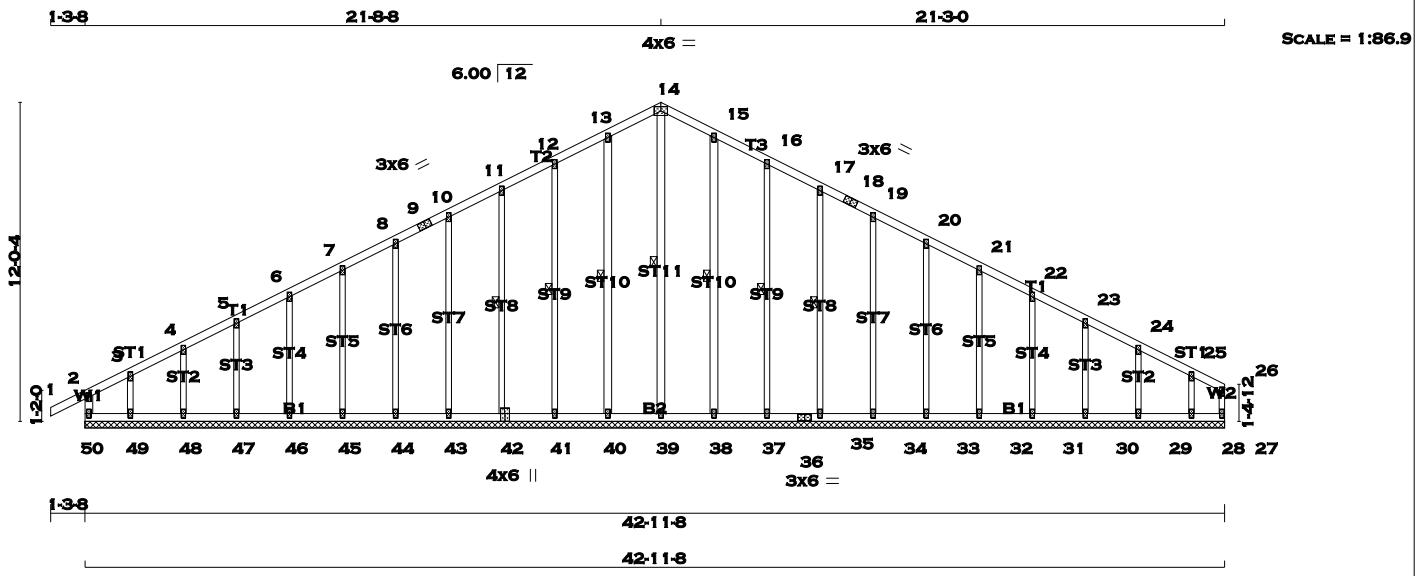
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.40 (26) (INPUT = 0.90)
JSI METAL= 0.05 (42) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

KOTT

CONTINUED ON PAGE 2



TOTAL WEIGHT = 224 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
50 - 2	2x4	DRY	No.2	SPF
1 - 9	2x4	DRY	No.2	SPF
9 - 14	2x4	DRY	No.2	SPF
14 - 18	2x4	DRY	No.2	SPF
18 - 26	2x4	DRY	No.2	SPF
27 - 26	2x3	DRY	No.2	SPF
50 - 42	2x4	DRY	No.2	SPF
42 - 36	2x4	DRY	No.2	SPF
36 - 27	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
39 - 14	2x4	DRY	No.2	SPF
40 - 13	2x4	DRY	No.2	SPF
38 - 15	2x4	DRY	No.2	SPF
ALL GABLE WEBS				
	2x3	DRY	No.2	SPF
EXCEPT				
ST1	2x4	DRY	No.2	SPF
ST2	2x4	DRY	No.2	SPF
ST12	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMV+p	MT20	2.0	4.0		
3, 4, 5, 6, 7, 8, 10, 11, 12, 13, 15, 16, 17, 19, 20, 21, 22, 23, 24, 25						
3	TMW+w	MT20	2.0	4.0		
9	TS-t	MT20	3.0	6.0		
14	TTW-p	MT20	4.0	6.0		
18	TS-t	MT20	3.0	6.0		
26	TMV+p	MT20	2.0	4.0		
27	BMV1+p	MT20	2.0	4.0		
28, 29, 30, 31, 32, 33, 34, 35, 37, 38, 39, 40, 41, 43, 44, 45, 46, 47, 48, 49						
28	BMW1+w	MT20	2.0	4.0		
36	BS-t	MT20	3.0	6.0		
42	BSW1+l	MT20	4.0	6.0		
50	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 14-39, 13-40, 12-41, 11-42, 15-38, 16-37, 17-35. DBS = 20-0-0 . CBF = 21 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
50- 2	-202 / 0	0.0	0.0	0.03 (1)	7.81
1- 2	0 / 23	-77.3	-77.3	0.10 (1)	10.00
2- 3	-24 / 0	-77.3	-77.3	0.08 (1)	6.25
3- 4	-3 / 0	-77.3	-77.3	0.04 (1)	10.00
4- 5	-3 / 0	-77.3	-77.3	0.04 (1)	10.00
5- 6	0 / 1	-77.3	-77.3	0.04 (1)	10.00
6- 7	0 / 3	-77.3	-77.3	0.04 (1)	10.00
7- 8	0 / 5	-77.3	-77.3	0.04 (1)	10.00
8- 9	0 / 7	-77.3	-77.3	0.04 (1)	10.00
9-10	0 / 7	-77.3	-77.3	0.04 (1)	10.00
10-11	0 / 8	-77.3	-77.3	0.04 (1)	10.00
11-12	0 / 12	-77.3	-77.3	0.04 (1)	10.00
12-13	0 / 14	-77.3	-77.3	0.04 (1)	10.00
13-14	0 / 14	-77.3	-77.3	0.04 (1)	10.00
14-15	0 / 14	-77.3	-77.3	0.04 (1)	10.00
15-16	0 / 15	-77.3	-77.3	0.04 (1)	10.00
16-17	0 / 12	-77.3	-77.3	0.04 (1)	10.00
17-18	0 / 11	-77.3	-77.3	0.04 (1)	6.25
18-19	0 / 11	-77.3	-77.3	0.04 (1)	6.25
19-20	0 / 9	-77.3	-77.3	0.04 (1)	10.00
20-21	0 / 8	-77.3	-77.3	0.04 (1)	10.00
21-22	0 / 6	-77.3	-77.3	0.04 (1)	10.00
22-23	0 / 3	-77.3	-77.3	0.04 (1)	10.00
23-24	0 / 0	-77.3	-77.3	0.04 (1)	10.00
24-25	-2 / 0	-77.3	-77.3	0.04 (1)	10.00
25-26	-4 / 0	-77.3	-77.3	0.03 (1)	10.00
27-26	-38 / 0	0.0	0.0	0.01 (1)	7.81
50-49	0 / 10	-17.5	-17.5	0.03 (1)	10.00
49-48	0 / 6	-17.5	-17.5	0.01 (4)	10.00
48-47	0 / 2	-17.5	-17.5	0.01 (4)	10.00
47-46	0 / 0	-17.5	-17.5	0.01 (4)	10.00
46-45	-3 / 0	-17.5	-17.5	0.01 (4)	10.00
45-44	-5 / 0	-17.5	-17.5	0.01 (4)	10.00
44-43	-6 / 0	-17.5	-17.5	0.02 (4)	10.00
43-42	-8 / 0	-17.5	-17.5	0.02 (4)	10.00
42-41	-11 / 0	-17.5	-17.5	0.01 (4)	6.25
41-40	-12 / 0	-17.5	-17.5	0.01 (4)	6.25
40-39	-13 / 0	-17.5	-17.5	0.01 (4)	6.25
39-38	-13 / 0	-17.5	-17.5	0.01 (4)	6.2
38-37	-12 / 0	-17.5	-17.5	0.01 (4)	6.2
37-36	-11 / 0	-17.5	-17.5	0.01 (4)	6.2
36-35	-11 / 0	-17.5	-17.5	0.01 (4)	6.2
35-34	-10 / 0	-17.5	-17.5	0.01 (4)	10.0
34-33	-8 / 0	-17.5	-17.5	0.01 (4)	10.0

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
39-14	-169 / 0	0.13 (1)
40-13	-161 / 0	0.10 (1)
41-12	-150 / 0	0.10 (1)
42-11	-154 / 0	0.08 (1)
43-10	-154 / 0	0.19 (1)
44- 8	-154 / 0	0.13 (1)
45- 7	-154 / 0	0.09 (1)
46- 6	-154 / 0	0.06 (1)
47- 5	-151 / 0	0.04 (1)
48- 4	-163 / 0	0.03 (1)
49- 3	-103 / 0	0.02 (1)
38-15	-161 / 0	0.10 (1)
37-16	-150 / 0	0.10 (1)
35-17	-155 / 0	0.08 (1)
34-19	-154 / 0	0.19 (1)
33-20	-154 / 0	0.13 (1)
32-21	-154 / 0	0.09 (1)
31-22	-154 / 0	0.06 (1)
30-23	-152 / 0	0.04 (1)
29-24	-158 / 0	0.03 (1)
28-25	-129 / 0	0.02 (1)

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.10 (1-2:1) , BC=0.03 (49-50:1) , WB=0.19 (10-43:1) , SSI=0.07 (1-2:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

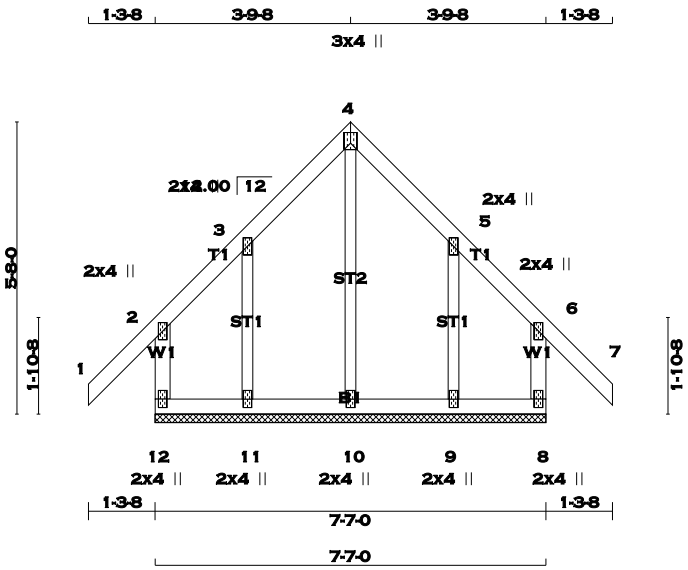
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.47 (27) (INPUT = 0.90)
JSI METAL= 0.05 (42) (INPUT = 1.00)

KOTT

CONTINUED ON PAGE 2



TOTAL WEIGHT = 38 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
12 - 2	2x4	DRY	No.2	SPF
1 - 4	2x4	DRY	No.2	SPF
4 - 7	2x4	DRY	No.2	SPF
8 - 6	2x4	DRY	No.2	SPF
12 - 8	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMV+p	MT20	2.0	4.0		
3	TMW+w	MT20	2.0	4.0		
4	TTW+p	MT20	3.0	4.0	Edge	
5	TMW+w	MT20	2.0	4.0		
6	TMV+p	MT20	2.0	4.0		
8	BMV1+p	MT20	2.0	4.0		
9, 10, 11						
9	BMW1+w	MT20	2.0	4.0		
12	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
12- 2	-174 / 0	0.0	0.0	0.04 (1)	7.81	10- 4	-214 / 0	0.10 (1)
1- 2	0 / 38	-77.3	-77.3	0.11 (1)	10.00	11- 3	-120 / 0	0.03 (1)
2- 3	-1 / 11	-77.3	-77.3	0.07 (1)	10.00	9- 5	-120 / 0	0.03 (1)
3- 4	0 / 33	-77.3	-77.3	0.05 (1)	10.00			
4- 5	0 / 33	-77.3	-77.3	0.05 (1)	10.00			
5- 6	-1 / 11	-77.3	-77.3	0.07 (1)	10.00			
6- 7	0 / 38	-77.3	-77.3	0.11 (1)	10.00			
8- 6	-174 / 0	0.0	0.0	0.04 (1)	7.81			
12-11	-16 / 0	-17.5	-17.5	0.01 (4)	6.25			
11-10	-20 / 0	-17.5	-17.5	0.01 (4)	6.25			
10- 9	-20 / 0	-17.5	-17.5	0.01 (4)	6.25			
9- 8	-16 / 0	-17.5	-17.5	0.01 (4)	6.25			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11 (1-2:1) , BC=0.01 (10-11:4) , WB=0.10 (4-10:1) , SSI=0.06 (1-2:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.34 (4) (INPUT = 0.90)
JSI METAL= 0.07 (4) (INPUT = 1.00)

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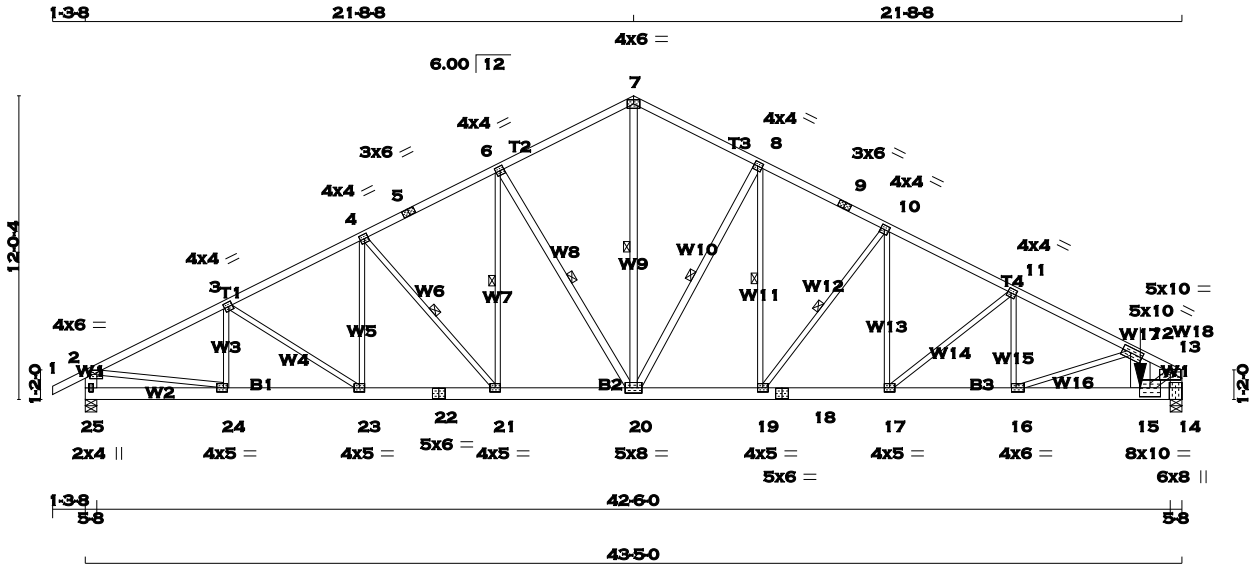
JUNIPER 9
BUILDING DIVISION

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

KOTT



TOTAL WEIGHT = 6 X 239 = 1433 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 5	2x4	DRY	No.2	SPF
5 - 7	2x4	DRY	No.2	SPF
7 - 9	2x4	DRY	No.2	SPF
9 - 13	2x4	DRY	No.2	SPF
25 - 2	2x6	DRY	No.2	SPF
14 - 13	2x6	DRY	No.2	SPF
25 - 22	2x6	DRY	No.2	SPF
22 - 18	2x6	DRY	No.2	SPF
18 - 14	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
6 - 20	2x4	DRY	No.2	SPF
20 - 7	2x4	DRY	No.2	SPF
20 - 8	2x4	DRY	No.2	SPF
15 - 12	2x10	DRY	No.2	SPF
15 - 13	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS		SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
1- 5	1	12	TOP
5- 7	1	12	TOP
7- 9	1	12	TOP
9- 13	1	12	TOP
25- 2	2	12	TOP
14- 13	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
25- 22	2	12	TOP
22- 18	2	12	TOP
18- 14	2	9	SIDE(249.2)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x4	1	6	
2x10	3	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
25	3322	0	3322	293	-881	5-8	5-8
14	12164	0	13220	0	-3568	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 25 FOR 881 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 14 FOR 3568 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 293 LBS FACTORED HORIZONTAL REACTION AT JOINT 25

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
25	2641	1552 / 0	489 / 0	0 / 0	210 / -1015	600 / 0	0 / 0
14	9720	6212 / 0	1858 / 0	0 / 0	2784 / -3982	2230 / 0	0 / 0
HORIZONTAL REACTIONS							
25	---	0 / 0	0 / 0	0 / 0	209 / -174	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 25, 14

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 3.14 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 4-21, 8-19. DBS = 16-0-0 . CBF = 92 LBS.
1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 6-21. DBS = 20-0-0 . CBF = 48 LBS.
1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 6-20, 7-20. DBS = 10-0-0 . CBF = 90 LBS.
1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 8-20, 10-19. DBS = 6-0-0 . CBF = 84 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) TO EACH PLY USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (18)

CHORDS MEMB. FR-TO	FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 36	-102.7	-102.7	0.05 (2)	10.00	24- 3	-537 / 240	0.04 (1)
2- 3	-4976 / 1305	-102.7	-102.7	0.23 (2)	4.99	3-23	-285 / 196	0.07 (2)
3- 4	-4889 / 1336	-102.7	-102.7	0.22 (2)	5.02	23- 4	-54 / 346	0.03 (5)
4- 5	-4358 / 1245	-102.7	-102.7	0.21 (2)	5.26	4-21	-859 / 367	0.13 (2)
5- 6	-4358 / 1245	-102.7	-102.7	0.21 (2)	5.26	21- 6	-200 / 772	0.06 (2)
6- 7	-3730 / 1161	-102.7	-102.7	0.19 (2)	5.58	6-20	-1345 / 559	0.25 (2)
7- 8	-3729 / 1165	-102.7	-102.7	0.17 (3)	5.61	20- 7	-861 / 2889	0.21 (14)
8- 9	-4752 / 1376	-102.7	-102.7	0.19 (3)	5.10	20- 8	-2252 / 807	0.42 (3)
9-10	-4752 / 1376	-102.7	-102.7	0.19 (3)	5.10	19- 8	-508 / 1834	0.14 (3)
10-11	-6019 / 1696	-102.7	-102.7	0.22 (3)	4.62	19-10	-2137 / 719	0.33 (3)
11-12	-8002 / 2253	-102.7	-102.7	0.29 (3)	4.06	17-10	-464 / 1796	0.13 (3)
12-13	-13906 / 3807	-102.7	-102.7	0.31 (2)	3.14	17-11	-2782 / 864	0.68 (3)
25- 2	-3230 / 900	0.0	0.0	0.07 (1)	7.81	16-11	-501 / 2151	0.16 (2)
14-13	-12197 / 3315	0.0	0.0	0.26 (3)	5.37	16-12	-5698 / 1567	0.78 (2)
						15-12	-1091 / 4504	0.12 (2)
25-24	-279 / 258	-27.5	-27.5	0.04 (4)	6.25	2-24	-1059 / 4515	0.34 (1)
24-23	-1328 / 4472	-27.5	-27.5	0.21 (1)	6.2			
23-22	-1164 / 4366	-27.5	-27.5	0.19 (1)	6.2			
22-21	-1164 / 4366	-27.5	-27.5	0.19 (1)	6.2			
21-20	-925 / 3905	-27.5	-27.5	0.18 (1)	6.2			
20-19	-875 / 4258	-27.5	-27.5	0.20 (1)	6.2			
19-18	-1199 / 5376	-27.5	-27.5	0.24 (1)	6.2			

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP	CH.	LL =	30.1	PSF
		DL =	5.0	PSF
BOT	CH.	LL =	10.0	PSF
		DL =	7.0	PSF
TOTAL	LOAD	=	52.1	PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder
START DISTANCE = 41-9-4
START SPAN CARRIED = 11-10-0
END DISTANCE = 43-5-0
END SPAN CARRIED = 11-10-0
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 100 % OF GSL.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 4 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR NOT USED

(80 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 30.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.45")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/180 (2.89")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.31 (12-13:2) , BC=0.73 (15-16:3) , WB=0.78 (12-16:2) , SSI=0.20 (14-15:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE M
RESPONSIBLE F
TRUSS MANUFA

NAIL VALUES
PLATE GRIP(DF (PSI)
MAX MII
MT20 618 35

EMENT TOL. = 0.250 inches
AT JON... = ...Deg

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MAR 29, 2017
JUNIPER 9
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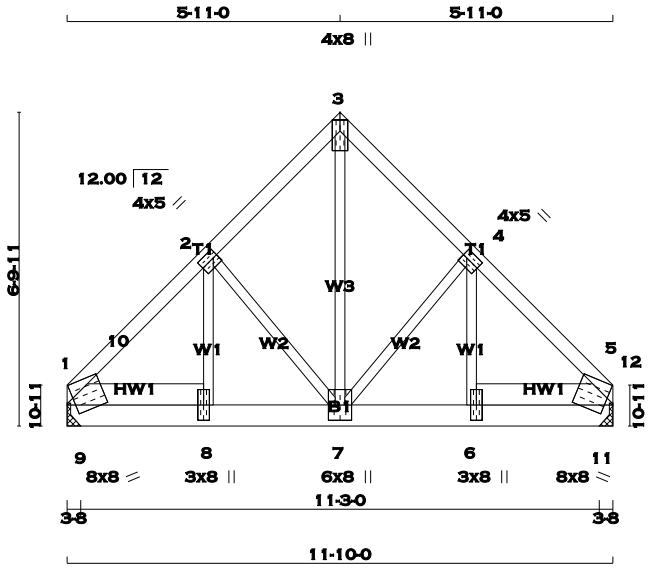
KOTT

CONTINUED ON PAGE 2



TOTAL WEIGHT = 3 X 68 = 205 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 3	2x4	DRY	No.2	SPF
3 - 5	2x4	DRY	No.2	SPF
1 - 5	2x6	DRY	No.2	SPF
REINFORCING MEMBERS				
HW1	2x6	DRY	No.2	SPF
HW2	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)		LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
1- 3	1	12	TOP
3- 5	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
1- 5	3	4	SIDE(956.3)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x6	2	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
1	TMBMW1-m	MT20	8.0	8.0	3.50	2.00
2	TMWW-t	MT20	4.0	5.0	2.00	1.25
3	TTW+p	MT20	4.0	8.0	Edge	
4	TMWW-t	MT20	4.0	5.0	2.00	1.25
5	TMBMW1-m	MT20	8.0	8.0	3.50	2.00
6	BMWW+t	MT20	3.0	8.0		
7	BMWWW+t	MT20	6.0	8.0		
8	BMWW+t	MT20	3.0	8.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
1	9258	0	9258	-229	-1706	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	
5	9258	0	9258	0	-1706	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

PROVIDE ANCHORAGE AT BEARING JOINT 1 FOR 1706 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 5 FOR 1706 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 229 LBS FACTORED HORIZONTAL REACTION AT JOINT 1

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
1	7279	4469 / 0	1277 / 0	0 / 0	1463 / -2204	1532 / 0	0 / 0
5	7279	4469 / 0	1277 / 0	0 / 0	1463 / -2204	1532 / 0	0 / 0
HORIZONTAL REACTIONS							
1	---	0 / 0	0 / 0	0 / 0	164 / -164	0 / 0	0 /0

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 3.81 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (18)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD FROM TO	LC1 MAX (PLF) CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1-10	-9748 / 1797	-102.7 -102.7	0.13 (2)	3.81	7- 3	-1724 / 8570	0.64 (1)
10- 2	-8705 / 1655	-102.7 -102.7	0.15 (3)	4.00	7- 4	-2730 / 695	0.30 (3)
2- 3	-6279 / 1309	-102.7 -102.7	0.12 (2)	4.61	6- 4	-623 / 3553	0.27 (2)
3- 4	-6279 / 1309	-102.7 -102.7	0.12 (3)	4.61	2- 7	-2730 / 695	0.30 (2)
4-12	-8705 / 1654	-102.7 -102.7	0.15 (2)	4.00	8- 2	-621 / 3553	0.27 (3)
12- 5	-9748 / 1782	-102.7 -102.7	0.13 (3)	3.81	9-10	-362 / 2018	0.00 (1)
					10- 8	-412 / 2416	0.10 (1)
1- 9	-848 / 3855	-1462.0-1462.0	0.16 (1)	6.25	11-12	-342 / 2018	0.00 (1)
9- 8	-848 / 3855	-1462.0-1462.0	0.39 (1)	6.25	12- 6	-423 / 2416	0.10 (1)
8- 7	-1168 / 6144	-1462.0-1462.0	0.46 (1)	6.25			
7- 6	-1087 / 6137	-1462.0-1462.0	0.46 (1)	6.25			
6-11	-704 / 3815	-1462.0-1462.0	0.39 (1)	6.25			
11- 5	-704 / 3815	-1462.0-1462.0	0.16 (1)	6.25			

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {26-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	30.1	PSF
	DL	=	5.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.1	PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 41-7-8
END DISTANCE = 11-10-0
END SPAN CARRIED = 41-7-8
END WALL WIDTH = 5-8
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 100 % OF GSL.

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 4 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR NOT USED

(80 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 30.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.39")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/180 (0.79")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.15 (4-12:2) , BC=0.46 (6-7:1) , WB=0.64 (3-7:1) , SSI=0.45 (8-9:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEME
PLATE ROTATIO
JSI GRIP= 0.83 (1
JSI METAL= 0.45

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MAR 29, 2017
JUNIPER 9
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READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

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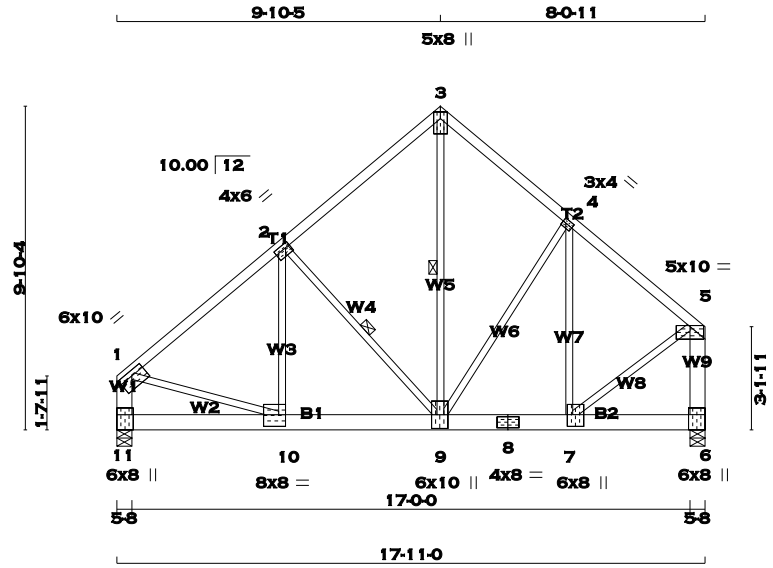
SCALE = 1:70.2



TOTAL WEIGHT = 3 X 101 = 303 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 3	2x4	DRY	No.2	SPF
3 - 5	2x4	DRY	No.2	SPF
11 - 1	2x6	DRY	No.2	SPF
6 - 5	2x6	DRY	No.2	SPF
11 - 8	2x6	DRY	2100F 1.8E	SPF
8 - 6	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
1- 3	1	12	TOP
3- 5	1	12	TOP
11- 1	2	12	TOP
6- 5	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
11- 8	3	4	SIDE(1023.2)
8- 6	3	4	SIDE(1023.2)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
1	TMVW-t	MT20	6.0	10.0	1.75	5.00
2	TMWW-t	MT20	4.0	6.0	1.50	1.75
3	TTW+p	MT20	5.0	8.0	Edge	
4	TMWW-t	MT20	3.0	4.0	1.50	1.25
5	TMVW-p	MT20	5.0	10.0	Edge	
6	BMV1+t	MT20	6.0	8.0	Edge	0.50
7	BMWW+t	MT20	6.0	8.0	4.25	2.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
11	14454	0	14868	389	-2658	5-8	5-8
6	14454	0	14874	0	-2670	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 11 FOR 2658 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 6 FOR 2670 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 389 LBS FACTORED HORIZONTAL REACTION AT JOINT 11

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
11	11364	6979 / 0	1993 / 0	0 / 0	3526 / -3436	2392 / 0	0 / 0
6	11364	6979 / 0	1993 / 0	0 / 0	3540 / -3445	2392 / 0	0 / 0
HORIZONTAL REACTIONS							
11	---	0 / 0	0 / 0	0 / 0	278 / -255	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 11, 6

BRACING
MAX. UNBRACED TOP CHORD LENGTH = 3.10 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x4 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 2-9, 3-9. DBS = 4-0-0 . CBF = 141 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) TO EACH PLY USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (18)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF) CSI (LC)	MAX. UNBRAC LENGTH
1- 2	-13776 / 2510	-102.7	-102.7	0.58 (2)	3.10
2- 3	-9452 / 1894	-102.7	-102.7	0.31 (2)	3.85
3- 4	-9438 / 1912	-102.7	-102.7	0.22 (3)	3.88
4- 5	-10433 / 1958	-102.7	-102.7	0.28 (3)	3.67
11- 1	-11631 / 2124	0.0	0.0	0.25 (1)	5.52
6- 5	-12164 / 2223	0.0	0.0	0.43 (1)	5.42
11-10	-367 / 346	-1510.8	-1510.8	0.40 (3)	6.25
10- 9	-1970 / 10658	-1510.8	-1510.8	0.56 (1)	6.25
9- 8	-1358 / 8024	-1510.8	-1510.8	0.39 (1)	6.25
8- 7	-1358 / 8024	-1510.8	-1510.8	0.39 (1)	6.25
7- 6	-23 / 49	-1510.8	-1510.8	0.28 (3)	6.25

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
10- 2	-1014 / 6104	0.45 (3)
2- 9	-5131 / 1194	0.58 (2)
9- 3	-2277 / 11654	0.85 (1)
9- 4	-1570 / 533	0.53 (3)
7- 4	-313 / 1625	0.12 (2)
1-10	-1893 / 10910	0.79 (1)
7- 5	-1675 / 9607	0.70 (1)

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {26-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	30.1	PSF
	DL	=	5.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.1	PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 42-11-8
END DISTANCE = 17-11-0
END SPAN CARRIED = 42-11-8
END WALL WIDTH = 5-8
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 100 % OF GSL.

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 4 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR NOT USED

(80 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 30.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.60")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/180 (1.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.58 (1-2:2) , BC=0.56 (9-10:1) , WB=0.85 (3-9:1) , SSI=0.90 (10-11:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEME
PLATE ROTATIO
JSI GRIP= 0.90 (2
JSI METAL= 0.86

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JUNIPER 9
BUILDING DIVISION

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

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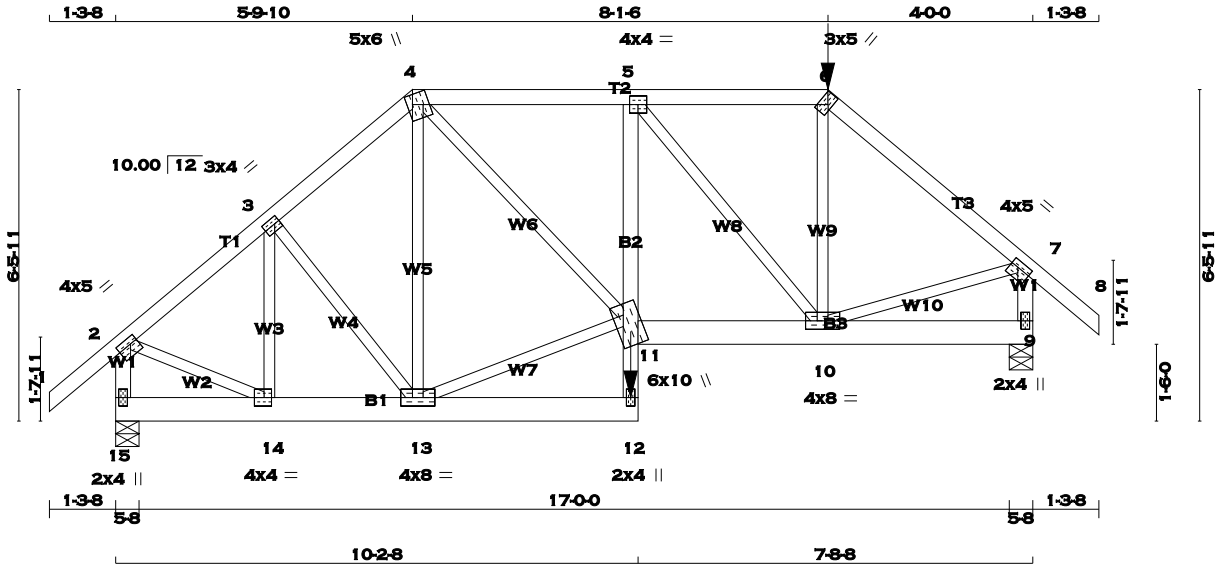

TOTAL WEIGHT = 102 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 6	2x4	DRY	No.2	SPF
6 - 8	2x4	DRY	No.2	SPF
15 - 2	2x4	DRY	No.2	SPF
9 - 7	2x4	DRY	No.2	SPF
15 - 12	2x6	DRY	No.2	SPF
12 - 5	2x4	DRY	No.2	SPF
11 - 9	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-t	MT20	4.0	5.0	1.50	1.75
3	TMWW-t	MT20	3.0	4.0	1.50	1.25
4	TTWW+m	MT20	5.0	6.0	2.25	1.25
5	TMVW-t	MT20	4.0	4.0		
6	TTW+h	MT20	3.0	5.0	2.50	1.00
7	TMVW-t	MT20	4.0	5.0	1.50	1.75
9	BMV1+p	MT20	2.0	4.0		
10	BMWWW-t	MT20	4.0	8.0		
11	BVMWW-w	MT20	6.0	10.0	3.50	7.50
12	BMV+p	MT20	2.0	4.0		
13	BMWWW-t	MT20	4.0	8.0		
14	BMWW-t	MT20	4.0	4.0	2.00	1.75
15	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 128.9 lbs FACTORED DOWN AT 13-11-0 ON TOP CHORD, AND 557.1 lbs FACTORED DOWN AT 10-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
15	1279	0	1279	0	0	5-8	5-8
9	1488	0	1488	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
15	895	642 / 0	0 / 0	0 / 0	0 / 0	253 / 0	0 / 0
9	1042	742 / 0	0 / 0	0 / 0	0 / 0	300 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 15, 9

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.80 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 34	-77.3	-77.3	0.12 (1)	10.00	14- 3	-325 / 0	0.08 (1)
2- 3	-1082 / 0	-77.3	-77.3	0.13 (1)	5.90	3-13	0 / 32	0.01 (1)
3- 4	-1145 / 0	-77.3	-77.3	0.13 (1)	5.77	13- 4	-290 / 0	0.19 (1)
4- 5	-1652 / 0	-77.3	-77.3	0.30 (1)	4.80	13-11	0 / 915	0.23 (1)
5- 6	-1050 / 0	-107.2	-107.2	0.27 (1)	5.76	4-11	0 / 1127	0.28 (1)
6- 7	-1358 / 0	-77.3	-77.3	0.28 (1)	5.22	5-10	-964 / 0	0.57 (1)
7- 8	0 / 34	-77.3	-77.3	0.12 (1)	10.00	10- 6	0 / 419	0.10 (1)
15- 2	-1244 / 0	0.0	0.0	0.14 (1)	7.17	2-14	0 / 910	0.23 (1)
9- 7	-1445 / 0	0.0	0.0	0.16 (1)	6.76	10- 7	0 / 1086	0.27 (1)
15-14	0 / 0	-17.5	-17.5	0.03 (1)	10.00			
14-13	0 / 843	-17.5	-17.5	0.14 (1)	10.00			
13-12	0 / 17	-17.5	-17.5	0.04 (4)	10.00			
12-11	0 / 583	0.0	0.0	0.15 (1)	10.00			
11- 5	0 / 309	0.0	0.0	0.10 (1)	10.00			
11-10	0 / 1664	-24.2	-24.2	0.24 (1)	10.00			
10- 9	0 / 0	-24.2	-24.2	0.05 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
6	13-11-0	-129	-129	---	FRONT	VERT	TOTAL
12	10-0-12	-557	-557	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP	CH.	LL =	23.3	PSF
		DL =	3.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.0	PSF
TOTAL	LOAD	=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 5-9-10
RIGHT SETBACK = 4-0-0
END SETBACK = 4-0-0
END WALL WIDTH = 5-8
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 7-7-12 OF SPAN MEASURED FROM THE RIGHT.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.60")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.60")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.30 (4-5:1) , BC=0.24 (10-11:1) , WB=0.57 (5-10:1) , SSI=0.21 (5-6:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE M
RESPONSIBLE F
TRUSS MANUFA

NAIL VALUES
PLATE GRIP(DF (PSI)
MAX MII
MT20 618 35

EMENT TOL. = 0.250 inches
AT ON L. = Deg

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

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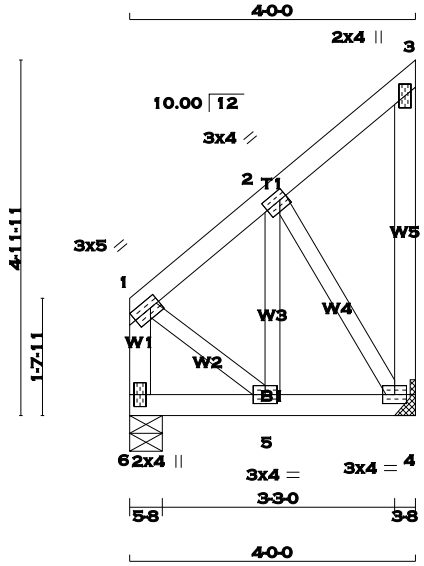


SCALE: 3/8"=1'

TOTAL WEIGHT = 24 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 3	2x4	DRY	No.2	SPF
4 - 3	2x4	DRY	No.2	SPF
6 - 1	2x4	DRY	No.2	SPF
6 - 4	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
1	TMVW-t	MT20	3.0	5.0	1.50	1.75
2	TMWW-t	MT20	3.0	4.0	1.50	1.25
3	TMV+p	MT20	2.0	4.0		
4	BMVW1-t	MT20	3.0	4.0		
5	BMWW-t	MT20	3.0	4.0		
6	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
4	557	0	557	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	
6	557	0	557	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
4	391	274 / 0	0 / 0	0 / 0	0 / 0	118 / 0	0 / 0
6	391	274 / 0	0 / 0	0 / 0	0 / 0	118 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 6

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	-284 / 0	-77.3	-77.3	0.05 (1)	6.25	5- 2	0 / 337	0.08 (1)
2- 3	-11 / 0	-77.3	-77.3	0.05 (1)	6.25	2- 4	-405 / 0	0.10 (1)
4- 3	-61 / 0	0.0	0.0	0.02 (1)	7.81	1- 5	0 / 270	0.07 (1)
6- 1	-397 / 0	0.0	0.0	0.05 (1)	7.81			
6- 5	0 / 0	-201.3	-201.3	0.12 (1)	10.00			
5- 4	0 / 226	-201.3	-201.3	0.16 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 10-2-8
END DISTANCE = 4-0-0
END SPAN CARRIED = 10-2-8
END WALL WIDTH = 5-8
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.05 (1-2:1) , BC=0.16 (4-5:1) , WB=0.10 (2-4:1) , SSI=0.21 (5-6:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

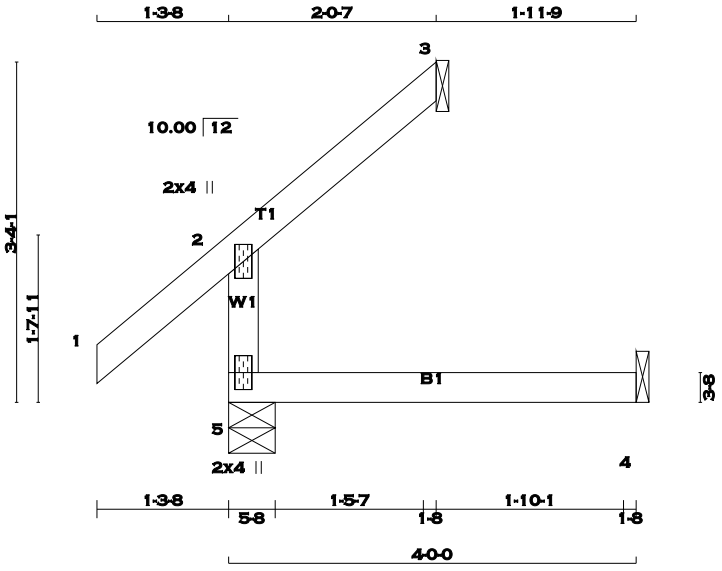
JSI GRIP= 0.67 (5) (INPUT = 0.90)
JSI METAL= 0.12 (5) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
R.M. TREVISAN
100136551
March 23rd, 2017
PROVINCE OF ONTARIO

RECEIVED
TOWN OF MILTON
MAR 29, 2017
JUNIPER 9
BUILDING DIVISION

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

KOTT



TOTAL WEIGHT = 11 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
5 - 2	2x4	DRY	No.2	SPF
1 - 3	2x4	DRY	No.2	SPF
5 - 4	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
2	TMV+p	MT20	2.0	4.0	
5	BMV1+p	MT20	2.0	4.0	

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
5	244	0	244	0	0	5-8	5-8
3	60	0	60	0	0	1-8	1-8
4	31	0	34	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) 3 , 4

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
5	171	123 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
3	41	36 / 0	0 / 0	0 / 0	0 / 0	5 / 0	0 / 0
4	24	0 / 0	0 / 0	0 / 0	0 / 0	24 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 5, 3

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
5- 2	-205 / 0	0.0	0.0	0.04 (4)	7.81			
1- 2	0 / 34	-77.3	-77.3	0.11 (1)	10.00			
2- 3	-12 / 0	-77.3	-77.3	0.05 (1)	6.25			
5- 4	0 / 0	-17.5	-17.5	0.06 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.11 (1-2:1) , BC=0.06 (4-5:4) , WB=0.00 (n/a:0) , SSI=0.06 (1-2:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

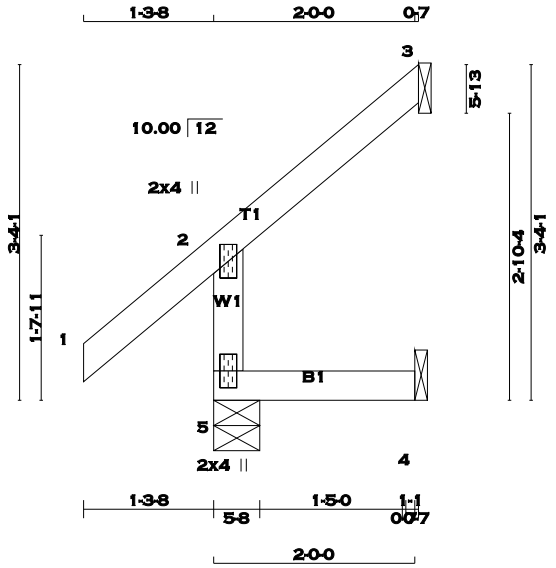
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (2) (INPUT = 0.90)
JSI METAL= 0.06 (2) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

KOTT



SCALE = 1:22.9

TOTAL WEIGHT = 9 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
5 - 2	2x4	DRY	No.2	SPF
1 - 3	2x4	DRY	No.2	SPF
5 - 4	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
2	TMV+p	MT20	2.0	4.0	
5	BMV1+p	MT20	2.0	4.0	

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
5	224	0	224	0	0	5-8	5-8
3	60	0	60	0	0	1-8	1-8
4	16	0	18	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) 3 , 4

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
5	155	123 / 0	0 / 0	0 / 0	0 / 0	31 / 0	0 / 0
3	41	36 / 0	0 / 0	0 / 0	0 / 0	5 / 0	0 / 0
4	13	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 5

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
5- 2	-205 / 0	0.0	0.0	0.01 (4)	7.81			
1- 2	0 / 34	-77.3	-77.3	0.11 (1)	10.00			
2- 3	-12 / 0	-77.3	-77.3	0.05 (1)	6.25			
5- 4	0 / 0	-17.5	-17.5	0.02 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.11 (1-2:1) , BC=0.02 (4-5:4) , WB=0.00 (n/a:0) , SSI=0.06 (1-2:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (2) (INPUT = 0.90)
JSI METAL= 0.06 (2) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
R.M. TREVISAN
100136551
March 23rd, 2017
PROVINCE OF ONTARIO

RECEIVED
TOWN OF MILTON
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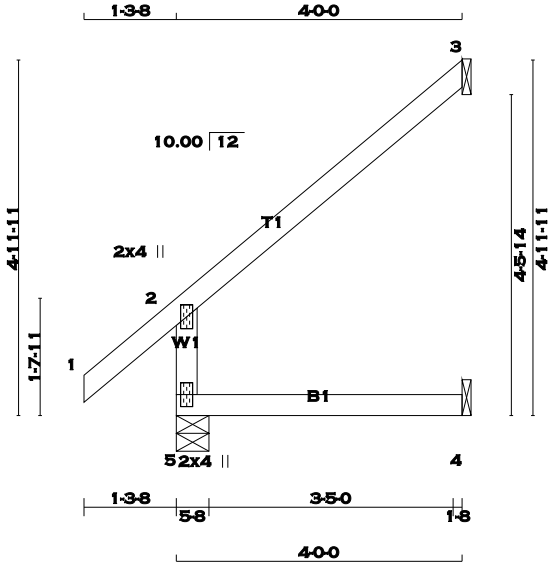
KOTT



SCALE: 3/8"=1'

TOTAL WEIGHT = 2 X 14 = 28 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
5 - 2	2x4	DRY	No.2	SPF
1 - 3	2x4	DRY	No.2	SPF
5 - 4	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
2	TMV+p	MT20	2.0	4.0	
5	BMV1+p	MT20	2.0	4.0	

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
5	340	0	340	0	0	5-8	5-8
3	116	0	116	0	0	1-8	1-8
4	31	0	34	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) 3 , 4

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
5	236	181 / 0	0 / 0	0 / 0	0 / 0	55 / 0	0 / 0
3	79	70 / 0	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0
4	24	0 / 0	0 / 0	0 / 0	0 / 0	24 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 5

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
5- 2	-300 / 0	0.0	0.0	0.04 (4)	7.81			
1- 2	0 / 34	-77.3	-77.3	0.11 (1)	10.00			
2- 3	-25 / 0	-77.3	-77.3	0.21 (1)	6.25			
5- 4	0 / 0	-17.5	-17.5	0.06 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.21 (2-3:1) , BC=0.06 (4-5:4) , WB=0.00 (n/a:0) , SSI=0.12 (2-3:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (2) (INPUT = 0.90)
JSI METAL= 0.08 (2) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
R.M. TREVISAN
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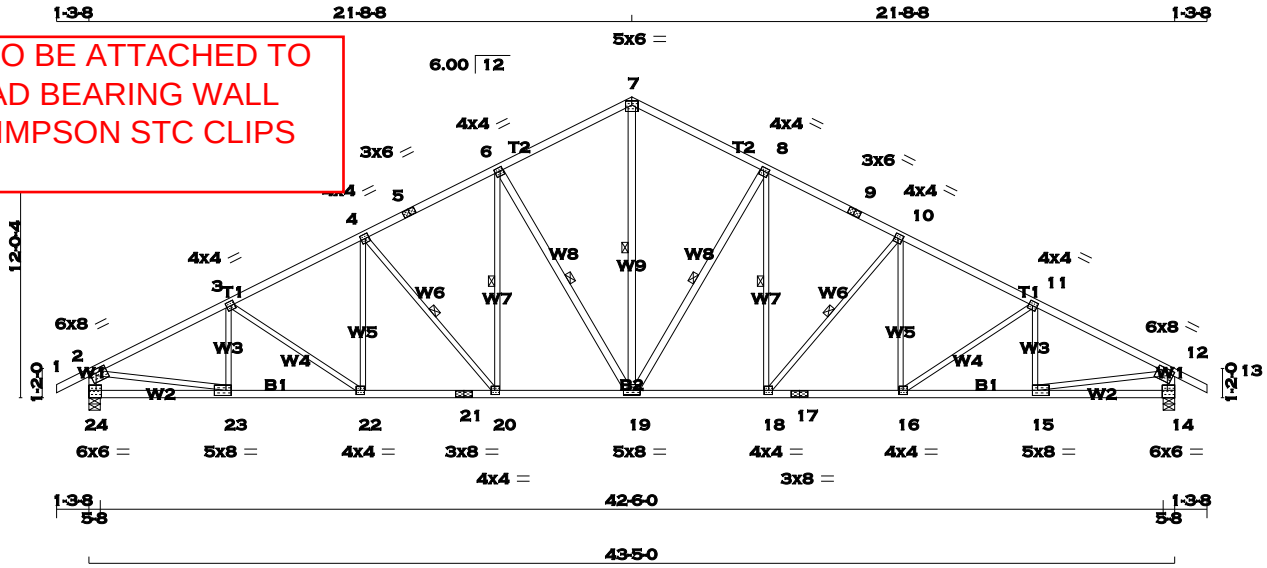
KOTT

TRUSS TO BE ATTACHED TO NON LOAD BEARING WALL USING SIMPSON STC CLIPS OR EQ.



TOTAL WEIGHT = 3 X 208 = 623 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 5	2x4	DRY	No.2	SPF
5 - 7	2x4	DRY	No.2	SPF
7 - 9	2x4	DRY	No.2	SPF
9 - 13	2x4	DRY	No.2	SPF
24 - 2	2x4	DRY	No.2	SPF
14 - 12	2x4	DRY	No.2	SPF
24 - 21	2x4	DRY	No.2	SPF
21 - 17	2x4	DRY	No.2	SPF
17 - 14	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
19 - 7	2x4	DRY	No.2	SPF
19 - 8	2x4	DRY	No.2	SPF
6 - 19	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-t	MT20	6.0	8.0	1.75	3.00
3, 4, 10, 11						
3	TMWW-t	MT20	4.0	4.0	2.00	1.75
5	TS-t	MT20	3.0	6.0		
6	TMWW-t	MT20	4.0	4.0	1.75	1.50
7	TTW-p	MT20	5.0	6.0	2.00	3.00
8	TMWW-t	MT20	4.0	4.0	1.75	1.50
9	TS-t	MT20	3.0	6.0		
12	TMVW-t	MT20	6.0	8.0	1.75	3.00
14	BMV1-t	MT20	6.0	6.0	Edge	2.50
15	BMWW-t	MT20	5.0	8.0	2.50	2.50
16	BMWW-t	MT20	4.0	4.0		
17	BS-t	MT20	3.0	8.0		
18	BMWW-t	MT20	4.0	4.0	2.00	1.75
19	BMWWW-t	MT20	5.0	8.0	2.25	4.00
20	BMWW-t	MT20	4.0	4.0	2.00	1.75
21	BS-t	MT20	3.0	8.0		
22	BMWW-t	MT20	4.0	4.0		
23	BMWW-t	MT20	5.0	8.0	2.50	2.50
24	BMV1-t	MT20	6.0	6.0	3.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
24	2966	0	2966	277	-771	5-8	5-8
14	2966	0	2966	0	-771	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 24 FOR 771 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 14 FOR 771 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 277 LBS FACTORED HORIZONTAL REACTION AT JOINT 24

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
24	2356	1387 / 0	434 / 0	0 / 0	103 / -894	535 / 0	0 / 0
14	2356	1387 / 0	434 / 0	0 / 0	103 / -894	535 / 0	0 / 0
HORIZONTAL REACTIONS							
24	---	0 / 0	0 / 0	0 / 0	198 / -198	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 24, 14

BRACING
MAX. UNBRACED TOP CHORD LENGTH = 2.73 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 5.94 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 7-19. DBS = 12-0-0 . CBF = 82 LBS.
1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 8-19, 6-19. DBS = 10-0-0 . CBF = 86 LBS.
1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 8-18, 6-20. DBS = 20-0-0 . CBF = 51 LBS.
1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 10-18, 4-20. DBS = 16-0-0 . CBF = 88 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (18)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 36	-102.7	-102.7	0.15 (2)	10.00	19- 7	-644 / 2187	0.50 (14)
2- 3	-4294 / 1101	-102.7	-102.7	0.97 (2)	2.73	19- 8	-1383 / 572	0.81 (3)
3- 4	-4139 / 1111	-102.7	-102.7	0.89 (1)	2.81	18- 8	-214 / 813	0.18 (3)
4- 5	-3604 / 1017	-102.7	-102.7	0.80 (1)	3.06	18-10	-878 / 373	0.41 (3)
5- 6	-3604 / 1017	-102.7	-102.7	0.80 (1)	3.06	16-10	-62 / 359	0.08 (6)
6- 7	-2961 / 929	-102.7	-102.7	0.69 (1)	3.44	16-11	-372 / 221	0.28 (3)
7- 8	-2961 / 929	-102.7	-102.7	0.69 (1)	3.44	15-11	-460 / 215	0.11 (1)
8- 9	-3604 / 1017	-102.7	-102.7	0.80 (1)	3.06	6-19	-1383 / 572	0.81 (2)
9-10	-3604 / 1017	-102.7	-102.7	0.80 (1)	3.06	20- 6	-214 / 813	0.18 (2)
10-11	-4139 / 1111	-102.7	-102.7	0.89 (1)	2.81	4-20	-878 / 373	0.41 (2)
11-12	-4294 / 1101	-102.7	-102.7	0.97 (3)	2.73	22- 4	-62 / 359	0.08 (5)
12-13	0 / 36	-102.7	-102.7	0.15 (3)	10.00	3-22	-372 / 221	0.28 (2)
24- 2	-2897 / 796	0.0	0.0	0.29 (1)	5.07	23- 3	-460 / 215	0.11 (1)
14-12	-2897 / 797	0.0	0.0	0.29 (1)	5.07	2-23	-875 / 3907	0.88 (1)
						15-12	-875 / 3907	0.88 (1)
24-23	-261 / 292	-27.5	-27.5	0.19 (17)	6.25			
23-22	-1126 / 3862	-27.5	-27.5	0.70 (1)	5.9			
22-21	-943 / 3695	-27.5	-27.5	0.67 (1)	6.2			
21-20	-943 / 3695	-27.5	-27.5	0.67 (1)	6.2			
20-19	-702 / 3231	-27.5	-27.5	0.62 (1)	6.2			
19-18	-540 / 3231	-27.5	-27.5	0.62 (1)	6.2			
18-17	-667 / 3695	-27.5	-27.5	0.67 (1)	6.2			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	30.1	PSF
	DL	=	5.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 4 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR NOT USED

(80 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 30.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.45")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.29")
ALLOWABLE DEFL.(TL)= L/180 (2.89")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.40")

CSI: TC=0.97 (11-12:3) , BC=0.70 (22-23:1) , WB=0.88 (2-23:1) , SSI=0.28 (2-3:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (15) (INPUT = 0.90)
JSI METAL= 0.96 (17) (INPUT = 1.00)

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MAR 29, 2017
JUNIPER 9
BUILDING DIVISION

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

KOTT

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
PT0317-179	T02	1	1	TRUSS DESC. LECCO RIDGE-JUNIPER 9 EL 2	Page 14 of 28 ENG JOB: PT0317-179

KOTT , Stouffville, ON, CGC

Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Thu Mar 23 11:07:40 2017 Page 1

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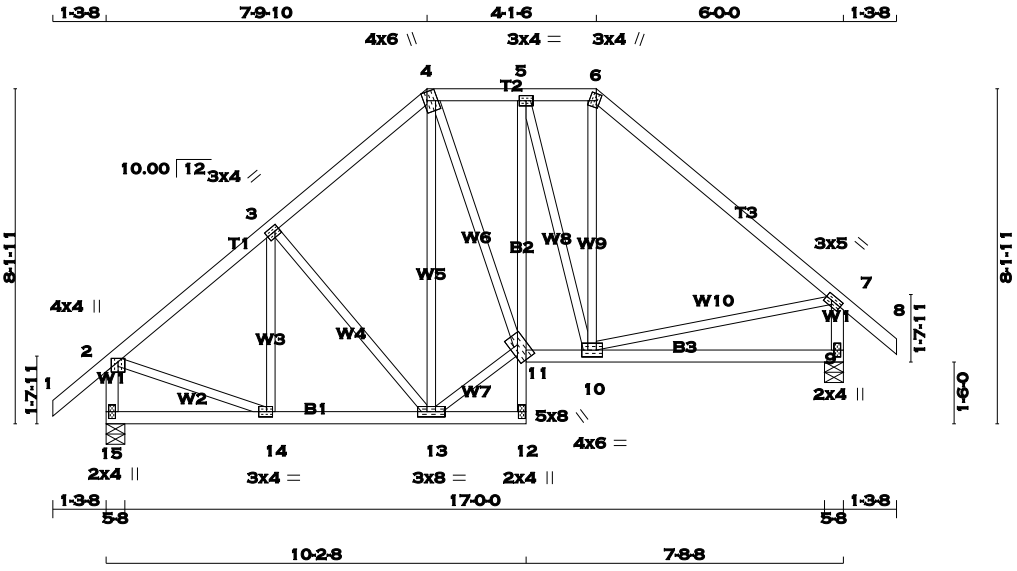


TOTAL WEIGHT = 98 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 6	2x4	DRY	No.2	SPF
6 - 8	2x4	DRY	No.2	SPF
15 - 2	2x4	DRY	No.2	SPF
9 - 7	2x4	DRY	No.2	SPF
15 - 12	2x4	DRY	No.2	SPF
12 - 5	2x3	DRY	No.2	SPF
11 - 9	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW+p	MT20	4.0	4.0	1.00	2.00
3	TMWW-t	MT20	3.0	4.0	1.50	1.25
4	TTWW+m	MT20	4.0	6.0	Edge	1.00
5	TMVW-t	MT20	3.0	4.0		
6	TTW+m	MT20	3.0	4.0	2.00	1.00
7	TMVW-t	MT20	3.0	5.0	1.50	1.75
9	BMV1+p	MT20	2.0	4.0		
10	BMWWW-t	MT20	4.0	6.0		
11	BVMWW-w	MT20	5.0	8.0	2.50	5.00
12	BMV+p	MT20	2.0	4.0		
13	BMWWW-t	MT20	3.0	8.0		
14	BMWW-t	MT20	3.0	4.0	1.50	1.75
15	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
15	957	0	957	0	0	5-8	5-8
9	957	0	957	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
15	669	482 / 0	0 / 0	0 / 0	0 / 0	187 / 0	0 / 0
9	669	482 / 0	0 / 0	0 / 0	0 / 0	187 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 15, 9

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 34	-77.3	-77.3	0.11 (1)	10.00	14- 3	-123 / 30	0.04 (1)
2- 3	-764 / 0	-77.3	-77.3	0.16 (1)	6.25	3-13	-195 / 0	0.12 (1)
3- 4	-649 / 0	-77.3	-77.3	0.15 (1)	6.25	13- 4	-158 / 0	0.20 (1)
4- 5	-614 / 0	-77.3	-77.3	0.05 (1)	6.25	13-11	0 / 602	0.14 (1)
5- 6	-551 / 0	-77.3	-77.3	0.03 (1)	6.25	4-11	0 / 390	0.09 (1)
6- 7	-714 / 0	-77.3	-77.3	0.37 (1)	6.25	5-10	-251 / 0	0.20 (1)
7- 8	0 / 34	-77.3	-77.3	0.11 (1)	10.00	10- 6	0 / 162	0.04 (4)
15- 2	-927 / 0	0.0	0.0	0.10 (1)	7.81	2-14	0 / 635	0.14 (1)
9- 7	-913 / 0	0.0	0.0	0.10 (1)	7.81	10- 7	0 / 560	0.13 (1)
15-14	0 / 0	-17.5	-17.5	0.06 (4)	10.00			
14-13	0 / 604	-17.5	-17.5	0.13 (1)	10.00			
13-12	0 / 3	-17.5	-17.5	0.04 (4)	10.00			
12-11	0 / 13	0.0	0.0	0.01 (1)	10.00			
11- 5	0 / 74	0.0	0.0	0.03 (1)	10.00			
11-10	0 / 615	-17.5	-17.5	0.20 (4)	10.00			
10- 9	0 / 0	-17.5	-17.5	0.15 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.60")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.60")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.37 (6-7:1) , BC=0.20 (10-11:4) , WB=0.20 (4-13:1) , SSI=0.14 (6-7:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

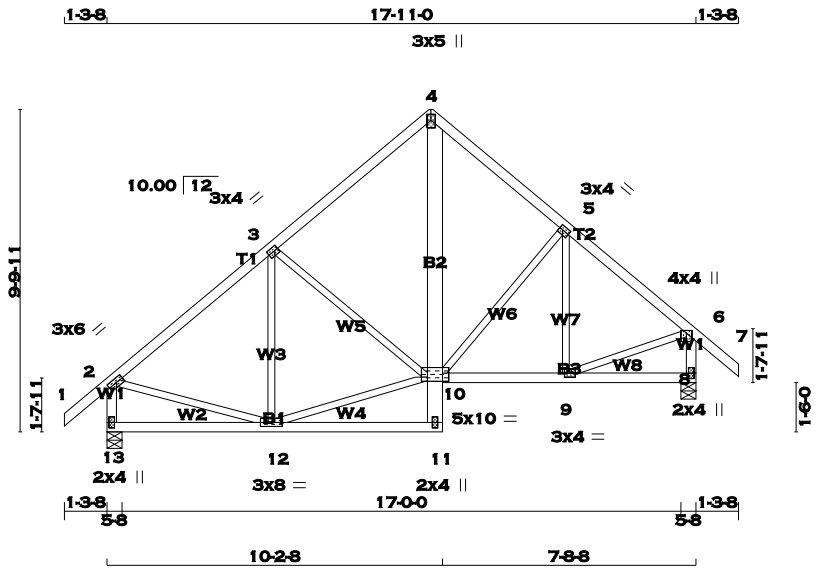
JSI GRIP= 0.90 (14) (INPUT = 0.90)
JSI METAL= 0.28 (7) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
R.M. TREVISAN
100136551
March 23rd, 2017
PROVINCE OF ONTARIO

RECEIVED
TOWN OF MILTON
MAR 29, 2017
JUNIPER 9
BUILDING DIVISION

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

KOTT



TOTAL WEIGHT = 99 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 7	2x4	DRY	No.2	SPF
13 - 2	2x4	DRY	No.2	SPF
8 - 6	2x4	DRY	No.2	SPF
13 - 11	2x4	DRY	No.2	SPF
11 - 4	2x6	DRY	No.2	SPF
10 - 8	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-t	MT20	3.0	6.0	1.50	2.75
3	TMWW-t	MT20	3.0	4.0	1.50	1.25
4	TTV+p	MT20	3.0	5.0	2.25	1.50
5	TMWW-t	MT20	3.0	4.0	1.50	1.25
6	TMVW+p	MT20	4.0	4.0	1.00	2.00
8	BMV1+p	MT20	2.0	4.0		
9	BMWW-t	MT20	3.0	4.0	1.50	1.75
10	BVMWWW-l	MT20	5.0	10.0	3.00	2.25
11	BMV+p	MT20	2.0	4.0		
12	BMWWW-t	MT20	3.0	8.0		
13	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
13	957	0	957	0	0	5-8	5-8
8	957	0	957	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
13	669	482 / 0	0 / 0	0 / 0	0 / 0	187 / 0	0 / 0
8	669	482 / 0	0 / 0	0 / 0	0 / 0	187 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 13, 8

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 34	-77.3	-77.3	0.11 (1)	10.00	12- 3	-230 / 22	0.12 (1)
2- 3	-765 / 0	-77.3	-77.3	0.25 (1)	6.25	3-10	-192 / 0	0.14 (1)
3- 4	-630 / 0	-77.3	-77.3	0.25 (1)	6.25	10- 5	-206 / 0	0.13 (1)
4- 5	-638 / 0	-77.3	-77.3	0.16 (1)	6.25	9- 5	-118 / 37	0.04 (1)
5- 6	-766 / 0	-77.3	-77.3	0.17 (1)	6.25	2-12	0 / 631	0.14 (1)
6- 7	0 / 34	-77.3	-77.3	0.11 (1)	10.00	9- 6	0 / 640	0.14 (1)
13- 2	-923 / 0	0.0	0.0	0.10 (1)	7.81	12-10	0 / 613	0.14 (1)
8- 6	-929 / 0	0.0	0.0	0.10 (1)	7.81			
13-12	0 / 0	-17.5	-17.5	0.12 (4)	10.00			
12-11	0 / 23	-17.5	-17.5	0.12 (4)	10.00			
11-10	0 / 37	0.0	0.0	0.03 (1)	10.00			
10- 4	0 / 515	0.0	0.0	0.08 (1)	10.00			
10- 9	0 / 607	-17.5	-17.5	0.13 (1)	10.00			
9- 8	0 / 0	-17.5	-17.5	0.07 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.60")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.60")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.25 (2-3:1) , BC=0.13 (9-10:1) , WB=0.14 (6-9:1) , SSI=0.14 (2-3:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

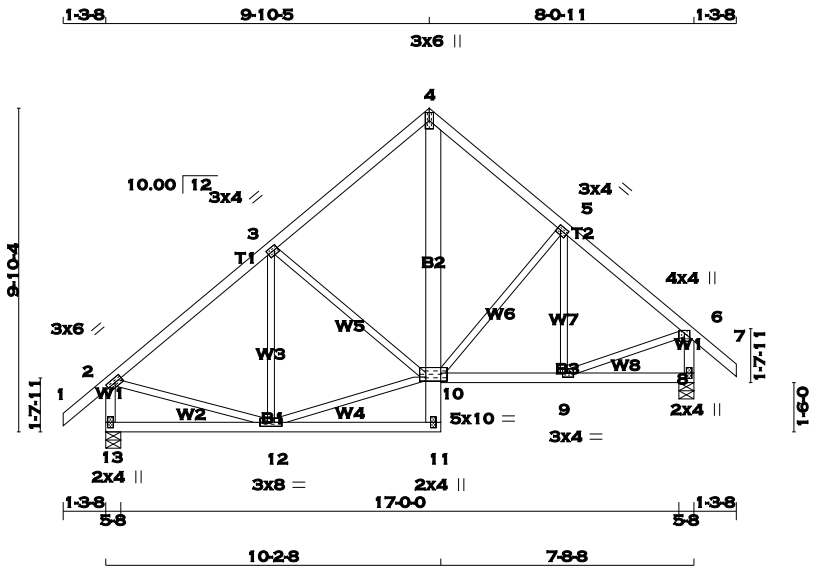
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (9) (INPUT = 0.90)
JSI METAL= 0.25 (6) (INPUT = 1.00)

RECEIVED
TOWN OF MILTON
MAR 29, 2017
JUNIPER 9
BUILDING DIVISION

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

KOTT



TOTAL WEIGHT = 99 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 7	2x4	DRY	No.2	SPF
13 - 2	2x4	DRY	No.2	SPF
8 - 6	2x4	DRY	No.2	SPF
13 - 11	2x4	DRY	No.2	SPF
11 - 4	2x6	DRY	No.2	SPF
10 - 8	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-t	MT20	3.0	6.0	1.50	2.75
3	TMWW-t	MT20	3.0	4.0	1.50	1.25
4	TTV+p	MT20	3.0	6.0		
5	TMWW-t	MT20	3.0	4.0	1.50	1.25
6	TMVW+p	MT20	4.0	4.0	1.00	2.00
8	BMV1+p	MT20	2.0	4.0		
9	BMWW-t	MT20	3.0	4.0	1.50	1.75
10	BVMWWW-l	MT20	5.0	10.0	3.00	2.25
11	BMV+p	MT20	2.0	4.0		
12	BMWWW-t	MT20	3.0	8.0		
13	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
13	957	0	957	0	0	5-8	5-8
8	957	0	957	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
13	669	482 / 0	0 / 0	0 / 0	0 / 0	187 / 0	0 / 0
8	669	482 / 0	0 / 0	0 / 0	0 / 0	187 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 13, 8

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
1- 2	0 / 34	-77.3	-77.3	0.11 (1)	10.00
2- 3	-764 / 0	-77.3	-77.3	0.25 (1)	6.25
3- 4	-630 / 0	-77.3	-77.3	0.25 (1)	6.25
4- 5	-638 / 0	-77.3	-77.3	0.16 (1)	6.25
5- 6	-766 / 0	-77.3	-77.3	0.17 (1)	6.25
6- 7	0 / 34	-77.3	-77.3	0.11 (1)	10.00
13- 2	-922 / 0	0.0	0.0	0.10 (1)	7.81
8- 6	-929 / 0	0.0	0.0	0.10 (1)	7.81
13-12	0 / 0	-17.5	-17.5	0.12 (4)	10.00
12-11	0 / 23	-17.5	-17.5	0.12 (4)	10.00
11-10	0 / 37	0.0	0.0	0.03 (1)	10.00
10- 4	0 / 516	0.0	0.0	0.08 (1)	10.00
10- 9	0 / 607	-17.5	-17.5	0.13 (1)	10.00
9- 8	0 / 0	-17.5	-17.5	0.07 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
12- 3	-229 / 22	0.12 (1)
3-10	-192 / 0	0.14 (1)
10- 5	-206 / 0	0.13 (1)
9- 5	-118 / 37	0.04 (1)
2-12	0 / 630	0.14 (1)
9- 6	0 / 640	0.14 (1)
12-10	0 / 613	0.14 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.60")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.60")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.25 (2-3:1) , BC=0.13 (9-10:1) , WB=0.14 (6-9:1) , SSI=0.14 (2-3:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (9) (INPUT = 0.90)
JSI METAL= 0.25 (6) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

KOTT

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
PT0317-179	**T05**	**1**	**1**	TRUSS DESC. LECCO RIDGE-JUNIPER 9 EL 2	Page 17 of 28 ENG JOB: PT0317-179

KOTT , Stouffville, ON, CGC

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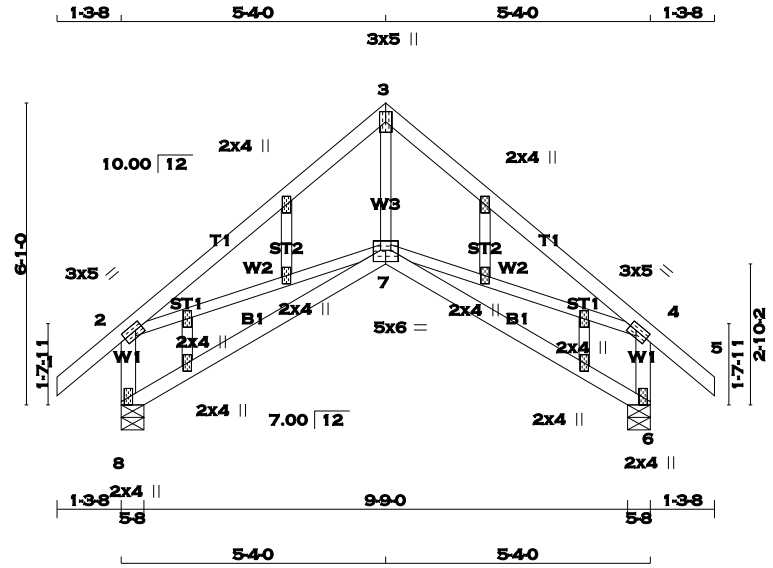

TOTAL WEIGHT = 52 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 3	2x4	DRY	No.2	SPF
3 - 5	2x4	DRY	No.2	SPF
8 - 2	2x4	DRY	No.2	SPF
6 - 4	2x4	DRY	No.2	SPF
8 - 7	2x4	DRY	No.2	SPF
7 - 6	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-t	MT20	3.0	5.0	1.50	1.75
3	TTW+p	MT20	3.0	5.0		
4	TMVW-t	MT20	3.0	5.0	1.50	1.75
6	BMV1+p	MT20	2.0	4.0	Edge	
7	BBWWW-p	MT20	5.0	6.0	2.75	3.00
8	BMV1+p	MT20	2.0	4.0	Edge	
9, 10, 11, 12, 13, 14, 15, 16						
9	NP+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
8	613	0	613	0	0	5-8	5-8
6	613	0	613	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
8	428	313 / 0	0 / 0	0 / 0	0 / 0	115 / 0	0 / 0
6	428	313 / 0	0 / 0	0 / 0	0 / 0	115 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 8, 6

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 34	-77.3	-77.3	0.11 (1)	10.00	7- 3	0 / 367	0.08 (1)
2- 3	-609 / 0	-77.3	-77.3	0.29 (1)	6.25	2- 7	0 / 488	0.11 (1)
3- 4	-609 / 0	-77.3	-77.3	0.29 (1)	6.25	7- 4	0 / 488	0.11 (1)
4- 5	0 / 34	-77.3	-77.3	0.11 (1)	10.00			
8- 2	-566 / 0	0.0	0.0	0.06 (1)	7.81			
6- 4	-566 / 0	0.0	0.0	0.06 (1)	7.81			
8- 7	0 / 0	-17.5	-17.5	0.15 (4)	10.00			
7- 6	0 / 0	-17.5	-17.5	0.15 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.36")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.36")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.29 (3-4:1) , BC=0.15 (7-8:4) , WB=0.11 (4-7:1) , SSI=0.12 (3-4:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (7) (INPUT = 0.90)
JSI METAL= 0.24 (4) (INPUT = 1.00)

RECEIVED
TOWN OF MILTON
MAR 29, 2017
JUNIPER 9
BUILDING DIVISION

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

KOTT

Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Thu Mar 23 11:07:42 2017 Page 1
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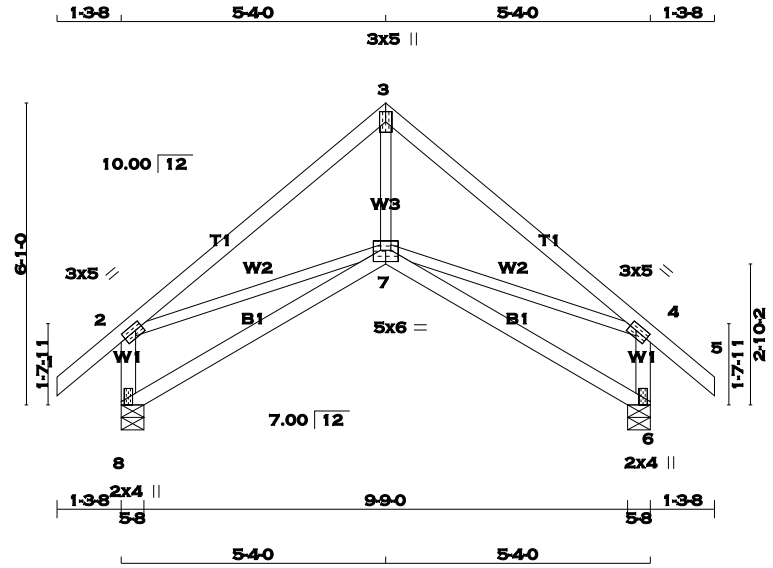


TOTAL WEIGHT = 48 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 3	2x4	DRY	No.2	SPF
3 - 5	2x4	DRY	No.2	SPF
8 - 2	2x4	DRY	No.2	SPF
6 - 4	2x4	DRY	No.2	SPF
8 - 7	2x4	DRY	No.2	SPF
7 - 6	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-t	MT20	3.0	5.0	1.50	1.75
3	TTW+p	MT20	3.0	5.0		
4	TMVW-t	MT20	3.0	5.0	1.50	1.75
6	BMV1+p	MT20	2.0	4.0	Edge	
7	BBWWW-p	MT20	5.0	6.0	2.75	3.00
8	BMV1+p	MT20	2.0	4.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
8	613	0	613	0	0	5-8	5-8
6	613	0	613	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
8	428	313 / 0	0 / 0	0 / 0	0 / 0	115 / 0	0 / 0
6	428	313 / 0	0 / 0	0 / 0	0 / 0	115 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 8, 6

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 34	-77.3	-77.3	0.11 (1)	10.00	7- 3	0 / 367	0.08 (1)
2- 3	-609 / 0	-77.3	-77.3	0.29 (1)	6.25	2- 7	0 / 488	0.11 (1)
3- 4	-609 / 0	-77.3	-77.3	0.29 (1)	6.25	7- 4	0 / 488	0.11 (1)
4- 5	0 / 34	-77.3	-77.3	0.11 (1)	10.00			
8- 2	-566 / 0	0.0	0.0	0.06 (1)	7.81			
6- 4	-566 / 0	0.0	0.0	0.06 (1)	7.81			
8- 7	0 / 0	-17.5	-17.5	0.15 (4)	10.00			
7- 6	0 / 0	-17.5	-17.5	0.15 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.36")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.36")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.29 (3-4:1) , BC=0.15 (7-8:4) , WB=0.11 (4-7:1) , SSI=0.12 (3-4:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

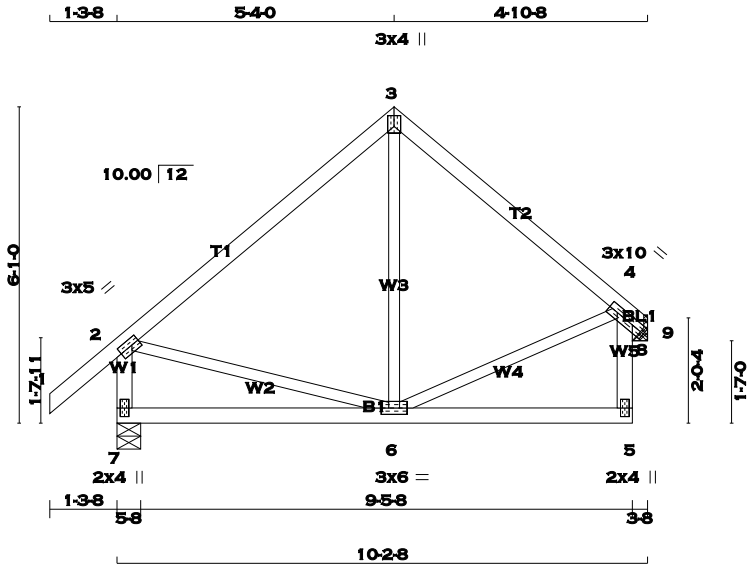
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (7) (INPUT = 0.90)
JSI METAL= 0.24 (4) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

KOTT



TOTAL WEIGHT = 2 X 45 = 90 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 3	2x4	DRY	No.2	SPF
3 - 4	2x4	DRY	No.2	SPF
7 - 2	2x4	DRY	No.2	SPF
5 - 4	2x4	DRY	No.2	SPF
7 - 5	2x4	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

BEARING NOTE: GAP BETWEEN INSIDE OF TOP CHORD BEARING AND FIRST DIAGONAL OR VERTICAL WEB SHALL NOT EXCEED 0.5 INCHES.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-t	MT20	3.0	5.0	1.50	1.75
3	TTW+p	MT20	3.0	4.0	2.50	1.50
4	TMVWK1-t	MT20	3.0	10.0	1.50	3.25
5	BMV+p	MT20	2.0	4.0		
6	BMWWW-t	MT20	3.0	6.0		
7	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
9(4)	463	0	463	0	0	HANGER BY OTHERS (** SEE "BEARING NOTE" **) MIN. SEAT SIZE: 3-8	
7	584	0	584	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
9(4)	325	228 / 0	0 / 0	0 / 0	0 / 0	98 / 0	0 / 0
7	408	299 / 0	0 / 0	0 / 0	0 / 0	109 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 7

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 34	-77.3	-77.3	0.11 (1)	10.00	6- 3	-22 / 78	0.03 (4)
2- 3	-282 / 0	-77.3	-77.3	0.28 (1)	6.25	2- 6	0 / 223	0.05 (1)
3- 4	-282 / 0	-77.3	-77.3	0.21 (1)	6.25	6- 4	0 / 197	0.04 (1)
7- 2	-547 / 0	0.0	0.0	0.06 (1)	7.81	4- 9	-483 / 0	0.05 (1)
5- 8	0 / 32	0.0	0.0	0.07 (1)	10.00	8- 9	0 / 137	0.00 (1)
8- 4	0 / 32	0.0	0.0	0.07 (1)	10.00			
7- 6	0 / 0	-17.5	-17.5	0.13 (4)	10.00			
6- 5	0 / 35	-17.5	-17.5	0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.33")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.33")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.28 (2-3:1) , BC=0.13 (5-6:4) , WB=0.05 (2-6:1) , SSI=0.12 (2-3:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

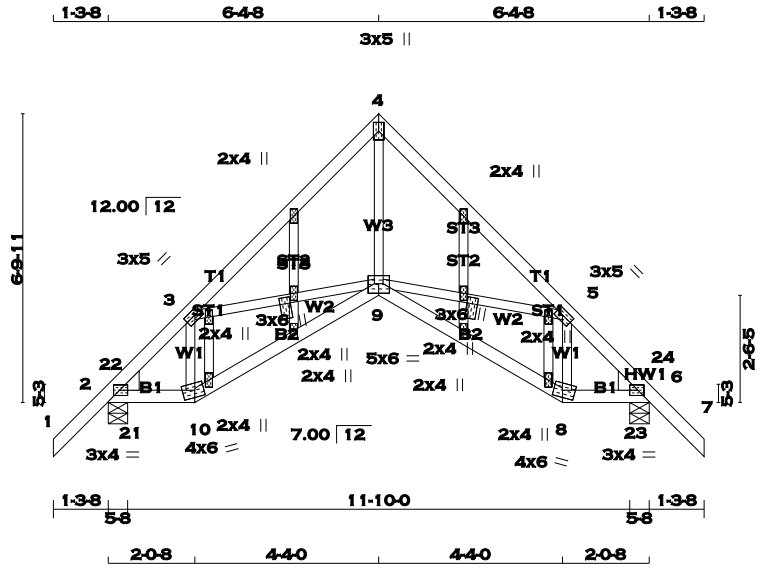
JSI GRIP= 0.48 (2) (INPUT = 0.90)
JSI METAL= 0.14 (2) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
R.M. TREVISAN
100136551
March 23rd, 2017
PROVINCE OF ONTARIO

RECEIVED
TOWN OF MILTON
MAR 29, 2017
JUNIPER 9
BUILDING DIVISION

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

KOTT



TOTAL WEIGHT = 63 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 7	2x4	DRY	No.2	SPF
2 - 10	2x4	DRY	No.2	SPF
10 - 9	2x4	DRY	No.2	SPF
9 - 8	2x4	DRY	No.2	SPF
8 - 6	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMBH1-l	MT20	3.0	4.0	1.50	2.25
3	TMWW-t	MT20	3.0	5.0	1.50	2.00
4	TTW+p	MT20	3.0	5.0		
5	TMWW-t	MT20	3.0	5.0	1.50	2.00
6	TMBH1-l	MT20	3.0	4.0	1.50	2.25
8	BBW-m	MT20	4.0	6.0	2.00	2.50
9	BBWWW-p	MT20	5.0	6.0	2.75	3.00
10	BBW-m	MT20	4.0	6.0	2.00	2.50
11	NP+t	MT20	3.0	6.0	1.75	0.25
11, 12, 13, 14, 15, 16, 17, 18, 19, 20						
11	NP+w	MT20	2.0	4.0		
18	NP+t	MT20	3.0	6.0	1.75	0.25

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
2	713	0	713	0	0	5-8	5-8	2x6 L
6	713	0	713	0	0	5-8	5-8	2x6 R

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
2	498	362 / 0	0 / 0	0 / 0	0 / 0	136 / 0	0 / 0
6	498	362 / 0	0 / 0	0 / 0	0 / 0	136 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 2, 6

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
1- 2	0 / 37	-77.3	-77.3	0.11 (1)	10.00	9- 4	0 / 663	0.15 (1)
2-22	-914 / 0	-77.3	-77.3	0.09 (1)	6.25	9- 5	-34 / 0	0.01 (1)
22- 3	-689 / 0	-77.3	-77.3	0.14 (1)	6.25	8- 5	-286 / 0	0.04 (1)
3- 4	-691 / 0	-77.3	-77.3	0.19 (1)	6.25	3- 9	-34 / 0	0.01 (1)
4- 5	-691 / 0	-77.3	-77.3	0.19 (1)	6.25	10- 3	-286 / 0	0.04 (1)
5-24	-689 / 0	-77.3	-77.3	0.14 (1)	6.25	21-22	0 / 228	0.00 (1)
24- 6	-914 / 0	-77.3	-77.3	0.09 (1)	6.25	23-24	0 / 228	0.00 (1)
6- 7	0 / 37	-77.3	-77.3	0.11 (1)	10.00			
2-21	0 / 523	-17.5	-17.5	0.18 (1)	10.00			
21-10	0 / 522	-17.5	-17.5	0.18 (1)	10.00			
10- 9	0 / 589	-17.5	-17.5	0.15 (1)	10.00			
9- 8	0 / 589	-17.5	-17.5	0.15 (1)	10.00			
8-23	0 / 522	-17.5	-17.5	0.18 (1)	10.00			
23- 6	0 / 523	-17.5	-17.5	0.18 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.19 (3-4:1) , BC=0.18 (2-21:1) , WB=0.15 (4-9:1) , SSI=0.14 (2-21:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.67 (6) (INPUT = 0.90)
JSI METAL= 0.22 (2) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
R.M. TREVISAN
100136551
March 23rd, 2017
PROVINCE OF ONTARIO

RECEIVED
TOWN OF MILTON
MAR 29, 2017
JUNIPER 9
BUILDING DIVISION

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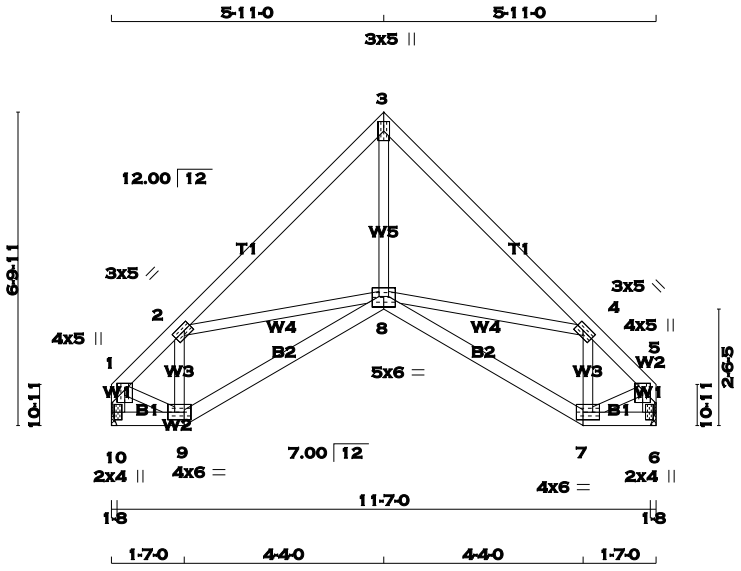
KOTT



TOTAL WEIGHT = 51 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 3	2x4	DRY	No.2	SPF
3 - 5	2x4	DRY	No.2	SPF
10 - 1	2x4	DRY	No.2	SPF
6 - 5	2x4	DRY	No.2	SPF
10 - 9	2x4	DRY	No.2	SPF
9 - 8	2x4	DRY	No.2	SPF
8 - 7	2x4	DRY	No.2	SPF
7 - 6	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
1	TMVW+p	MT20	4.0	5.0	1.75	2.00
2	TMWW-t	MT20	3.0	5.0	1.50	2.00
3	TTW+p	MT20	3.0	5.0		
4	TMWW-t	MT20	3.0	5.0	1.50	2.00
5	TMVW+p	MT20	4.0	5.0	1.75	2.00
6	BMV1+p	MT20	2.0	4.0		
7	BBWW-p	MT20	4.0	6.0	2.00	4.25
8	BBWWW-p	MT20	5.0	6.0	2.75	3.00
9	BBWW-p	MT20	4.0	6.0	2.00	4.25
10	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
10	561	0	561	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
6	561	0	561	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
10	394	276 / 0	0 / 0	0 / 0	0 / 0	118 / 0	0 / 0
6	394	276 / 0	0 / 0	0 / 0	0 / 0	118 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
1- 2	-553 / 0	-77.3	-77.3	0.15 (1)	6.25
2- 3	-623 / 0	-77.3	-77.3	0.18 (1)	6.25
3- 4	-623 / 0	-77.3	-77.3	0.18 (1)	6.25
4- 5	-553 / 0	-77.3	-77.3	0.15 (1)	6.25
10- 1	-547 / 0	0.0	0.0	0.05 (1)	7.81
6- 5	-547 / 0	0.0	0.0	0.05 (1)	7.81
10- 9	0 / 0	-17.5	-17.5	0.01 (4)	10.00
9- 8	0 / 487	-17.5	-17.5	0.13 (1)	10.00
8- 7	0 / 487	-17.5	-17.5	0.13 (1)	10.00
7- 6	0 / 0	-17.5	-17.5	0.01 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
8- 3	0 / 569	0.13 (1)
8- 4	0 / 13	0.00 (4)
7- 4	-341 / 0	0.05 (1)
2- 8	0 / 13	0.00 (4)
9- 2	-341 / 0	0.05 (1)
1- 9	0 / 461	0.10 (1)
7- 5	0 / 461	0.10 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.39")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.39")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.18 (2-3:1) , BC=0.13 (8-9:1) , WB=0.13 (3-8:1) , SSI=0.11 (2-3:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.66 (7) (INPUT = 0.90)
JSI METAL= 0.13 (1) (INPUT = 1.00)

RECEIVED
TOWN OF MILTON
MAR 29, 2017
JUNIPER 9
BUILDING DIVISION

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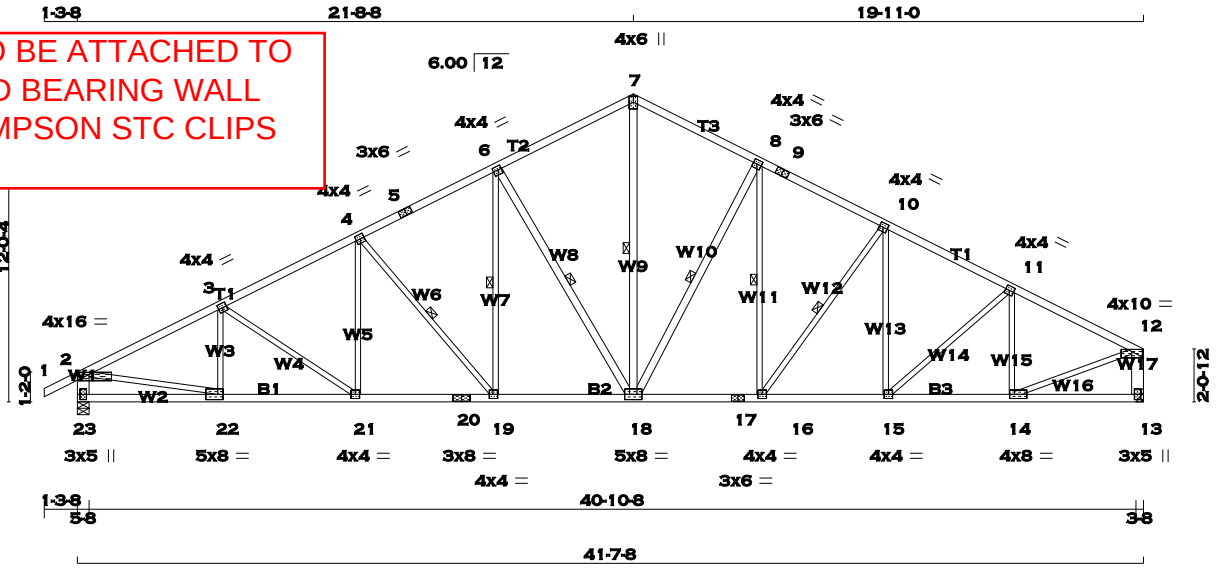
KOTT

TRUSS TO BE ATTACHED TO NON LOAD BEARING WALL USING SIMPSON STC CLIPS OR EQ.



TOTAL WEIGHT = 5 X 205 = 1026 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 5	2x4	DRY	No.2	SPF
5 - 7	2x4	DRY	No.2	SPF
7 - 9	2x4	DRY	No.2	SPF
9 - 12	2x4	DRY	No.2	SPF
23 - 2	2x6	DRY	No.2	SPF
13 - 12	2x6	DRY	No.2	SPF
23 - 20	2x4	DRY	No.2	SPF
20 - 17	2x4	DRY	No.2	SPF
17 - 13	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
6 - 18	2x4	DRY	No.2	SPF
18 - 7	2x4	DRY	No.2	SPF
18 - 8	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-p	MT20	4.0	16.0	1.00	5.50
3, 4, 8, 10, 11						
3	TMWW-t	MT20	4.0	4.0	2.00	1.75
5	TS-t	MT20	3.0	6.0		
6	TMWW-t	MT20	4.0	4.0	1.75	1.50
7	TTW+p	MT20	4.0	6.0	Edge	
9	TS-t	MT20	3.0	6.0		
12	TMVW-p	MT20	4.0	10.0	1.00	5.00
13	BMV1+p	MT20	3.0	5.0		
14	BMWW-t	MT20	4.0	8.0	2.00	2.25
15, 16, 21						
15	BMWW-t	MT20	4.0	4.0		
17	BS-t	MT20	3.0	6.0		
18	BMWWW-t	MT20	5.0	8.0		
19	BMWW-t	MT20	4.0	4.0	2.00	1.75
20	BS-t	MT20	3.0	8.0		
22	BMWW-t	MT20	5.0	8.0	2.50	2.50
23	BMV1+p	MT20	3.0	5.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
23	2849	0	2849	298	-748	5-8	5-8
13	2710	0	2710	0	-675	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

PROVIDE ANCHORAGE AT BEARING JOINT 23 FOR 748 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 13 FOR 675 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 298 LBS FACTORED HORIZONTAL REACTION AT JOINT 23

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
23	2263	1333 / 0	416 / 0	0 / 0	102 / -864	513 / 0	0 / 0
13	2167	1252 / 0	416 / 0	0 / 0	98 / -803	500 / 0	0 / 0
HORIZONTAL REACTIONS							
23	---	0 / 0	0 / 0	0 / 0	213 / -167	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 23

BRACING
MAX. UNBRACED TOP CHORD LENGTH = 2.82 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 5.97 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 4-19. DBS = 16-0-0 . CBF = 88 LBS.
1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 6-19, 8-16, 10-16. DBS = 20-0-0 . CBF = 74 LBS.
1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 6-18. DBS = 10-0-0 . CBF = 87 LBS.
1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 7-18. DBS = 14-0-0 . CBF = 87 LBS.
1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 8-18. DBS = 12-0-0 . CBF = 86 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (18)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 36	-102.7	-102.7	0.15 (2)	10.00	22- 3	-434 / 210	0.10 (1)
2- 3	-4095 / 1062	-102.7	-102.7	0.95 (2)	2.82	3-21	-387 / 226	0.30 (2)
3- 4	-3913 / 1067	-102.7	-102.7	0.85 (2)	2.91	21- 4	-65 / 367	0.08 (5)
4- 5	-3368 / 971	-102.7	-102.7	0.76 (1)	3.19	4-19	-885 / 376	0.41 (2)
5- 6	-3368 / 971	-102.7	-102.7	0.76 (1)	3.19	19- 6	-215 / 820	0.18 (2)
6- 7	-2719 / 865	-102.7	-102.7	0.65 (1)	3.59	6-18	-1390 / 573	0.81 (2)
7- 8	-2717 / 886	-102.7	-102.7	0.53 (1)	3.72	18- 7	-609 / 1989	0.47 (13)
8- 9	-3170 / 913	-102.7	-102.7	0.59 (1)	3.43	18- 8	-1143 / 501	0.66 (3)
9-10	-3170 / 913	-102.7	-102.7	0.59 (1)	3.43	16- 8	-171 / 609	0.14 (3)
10-11	-3450 / 944	-102.7	-102.7	0.64 (1)	3.25	16-10	-592 / 291	0.28 (3)
11-12	-3214 / 830	-102.7	-102.7	0.65 (3)	3.39	15-10	-99 / 162	0.09 (9)
23- 2	-2781 / 774	0.0	0.0	0.18 (1)	6.27	15-11	-33 / 315	0.07 (2)
13-12	-2649 / 700	0.0	0.0	0.20 (1)	6.39	14-11	-839 / 294	0.27 (1)
						2-22	-840 / 3728	0.84 (1)
23-22	-281 / 236	-27.5	-27.5	0.19 (17)	6.25	14-12	-681 / 3059	0.69 (1)
22-21	-1110 / 3684	-27.5	-27.5	0.67 (1)	5.97			
21-20	-923 / 3493	-27.5	-27.5	0.64 (1)	6.2			
20-19	-923 / 3493	-27.5	-27.5	0.64 (1)	6.2			
19-18	-680 / 3020	-27.5	-27.5	0.58 (1)	6.2			
18-17	-482 / 2842	-27.5	-27.5	0.55 (1)	6.2			
17-16	-482 / 2842	-27.5	-27.5	0.55 (1)	6.2			
16-15	-539 / 3079	-27.5	-27.5	0.55 (1)	6.2			

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	30.1	PSF
	DL	=	5.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 4 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR NOT USED

(80 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 30.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.39")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.24")
ALLOWABLE DEFL.(TL)= L/180 (2.78")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.34")

CSI: TC=0.95 (2-3:2) , BC=0.67 (21-22:1) , WB=0.84 (2-22:1) , SSI=0.28 (2-3:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (22) (INPUT = 0.90)
JSI METAL= 0.91 (20) (INPUT = 1.00)

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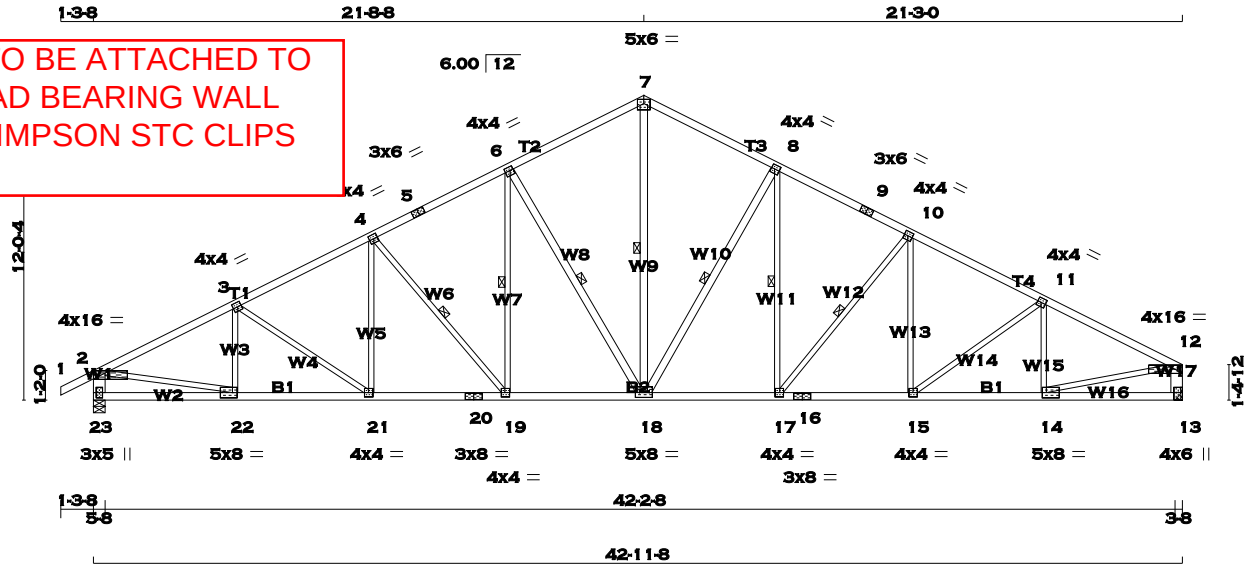
CONTINUED ON PAGE 2

TRUSS TO BE ATTACHED TO NON LOAD BEARING WALL USING SIMPSON STC CLIPS OR EQ.



TOTAL WEIGHT = 8 X 206 = 1651 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 5	2x4	DRY	No.2	SPF
5 - 7	2x4	DRY	No.2	SPF
7 - 9	2x4	DRY	No.2	SPF
9 - 12	2x4	DRY	No.2	SPF
23 - 2	2x6	DRY	No.2	SPF
13 - 12	2x6	DRY	No.2	SPF
23 - 20	2x4	DRY	No.2	SPF
20 - 16	2x4	DRY	No.2	SPF
16 - 13	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
6 - 18	2x4	DRY	No.2	SPF
18 - 7	2x4	DRY	No.2	SPF
18 - 8	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-p	MT20	4.0	16.0	1.00	5.50
3, 4, 10, 11						
3	TMWW-t	MT20	4.0	4.0	2.00	1.75
5	TS-t	MT20	3.0	6.0		
6	TMWW-t	MT20	4.0	4.0	1.75	1.50
7	TTW-p	MT20	5.0	6.0	2.00	3.00
8	TMWW-t	MT20	4.0	4.0	2.00	1.50
9	TS-t	MT20	3.0	6.0		
12	TMVW-p	MT20	4.0	16.0	1.00	Edge
13	BMV1+t	MT20	4.0	6.0	Edge	1.50
14	BMWW-t	MT20	5.0	8.0	2.50	2.00
15	BMWW-t	MT20	4.0	4.0		
16	BS-t	MT20	3.0	8.0		
17	BMWW-t	MT20	4.0	4.0	2.00	1.75
18	BMWWW-t	MT20	5.0	8.0	2.25	4.00
19	BMWW-t	MT20	4.0	4.0	2.00	1.75
20	BS-t	MT20	3.0	8.0		
21	BMWW-t	MT20	4.0	4.0		
22	BMWW-t	MT20	5.0	8.0	2.50	2.25
23	BMV1+p	MT20	3.0	5.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
23	2936	0	2936	294	-765	5-8	5-8
13	2797	0	2797	0	-707	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

PROVIDE ANCHORAGE AT BEARING JOINT 23 FOR 765 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 13 FOR 707 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 294 LBS FACTORED HORIZONTAL REACTION AT JOINT 23

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
23	2332	1373 / 0	430 / 0	0 / 0	103 / -886	529 / 0	0 / 0
13	2237	1292 / 0	430 / 0	0 / 0	108 / -837	516 / 0	0 / 0
HORIZONTAL REACTIONS							
23	---	0 / 0	0 / 0	0 / 0	210 / -170	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 23

BRACING
MAX. UNBRACED TOP CHORD LENGTH = 2.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 5.92 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 4-19. DBS = 16-0-0 . CBF = 88 LBS.
1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 6-19, 8-17. DBS = 20-0-0 . CBF = 51 LBS.
1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 6-18, 8-18. DBS = 10-0-0 . CBF = 87 LBS.
1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 7-18. DBS = 14-0-0 . CBF = 93 LBS.
1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 10-17. DBS = 18-0-0 . CBF = 90 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (18)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD FROM (PLF)	TO	LC1 MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 36	-102.7	-102.7	0.15 (2)	10.00	22- 3	-453 / 214	0.10 (1)
2- 3	-4243 / 1090	-102.7	-102.7	0.96 (2)	2.75	3-21	-376 / 222	0.29 (2)
3- 4	-4081 / 1100	-102.7	-102.7	0.88 (1)	2.83	21- 4	-63 / 361	0.08 (5)
4- 5	-3543 / 1004	-102.7	-102.7	0.79 (1)	3.09	4-19	-879 / 374	0.41 (2)
5- 6	-3543 / 1004	-102.7	-102.7	0.79 (1)	3.09	19- 6	-214 / 815	0.18 (2)
6- 7	-2900 / 912	-102.7	-102.7	0.68 (1)	3.47	6-18	-1385 / 572	0.81 (2)
7- 8	-2899 / 918	-102.7	-102.7	0.64 (1)	3.51	18- 7	-635 / 2136	0.49 (13)
8- 9	-3490 / 988	-102.7	-102.7	0.74 (1)	3.16	18- 8	-1320 / 552	0.77 (3)
9-10	-3490 / 988	-102.7	-102.7	0.74 (1)	3.16	17- 8	-201 / 759	0.17 (3)
10-11	-3952 / 1063	-102.7	-102.7	0.82 (1)	2.92	17-10	-798 / 349	0.38 (3)
11-12	-3980 / 1016	-102.7	-102.7	0.86 (3)	2.91	15-10	-45 / 305	0.07 (6)
23- 2	-2868 / 790	0.0	0.0	0.19 (1)	6.20	15-11	-246 / 181	0.19 (3)
13-12	-2730 / 733	0.0	0.0	0.18 (1)	6.32	14-11	-570 / 243	0.14 (1)
						2-22	-866 / 3861	0.87 (1)
23-22	-278 / 254	-27.5	-27.5	0.19 (17)	6.25	14-12	-835 / 3651	0.82 (1)
22-21	-1134 / 3816	-27.5	-27.5	0.69 (1)	5.9			
21-20	-950 / 3643	-27.5	-27.5	0.66 (1)	6.2			
20-19	-950 / 3643	-27.5	-27.5	0.66 (1)	6.2			
19-18	-708 / 3177	-27.5	-27.5	0.61 (1)	6.2			
18-17	-536 / 3129	-27.5	-27.5	0.61 (1)	6.2			
17-16	-653 / 3527	-27.5	-27.5	0.64 (1)	6.2			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	30.1	PSF
	DL	=	5.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 4 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR NOT USED

(80 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 30.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.27")
ALLOWABLE DEFL.(TL)= L/180 (2.86")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.38")

CSI: TC=0.96 (2-3:2) , BC=0.69 (21-22:1) , WB=0.87 (2-22:1) , SSI=0.28 (2-3:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (14) (INPUT = 0.90)
JSI METAL= 0.95 (20) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

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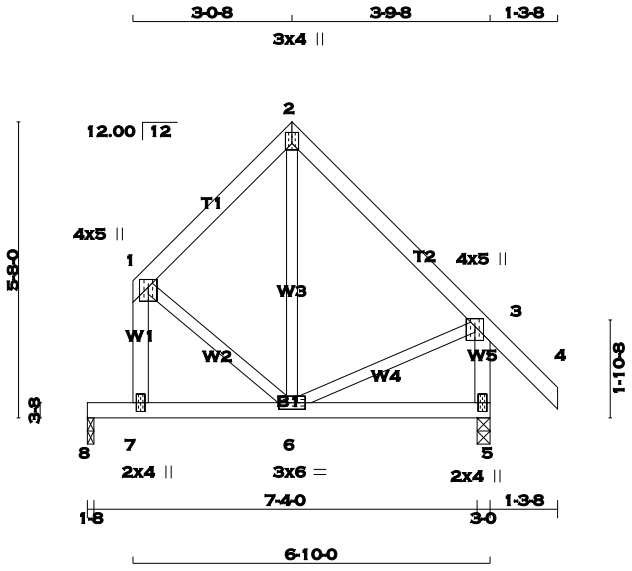

TOTAL WEIGHT = 2 X 37 = 73 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 2	2x4	DRY	No.2	SPF
2 - 4	2x4	DRY	No.2	SPF
7 - 1	2x4	DRY	No.2	SPF
5 - 3	2x4	DRY	No.2	SPF
8 - 5	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
1	TMVW+p	MT20	4.0	5.0	1.75	2.00
2	TTW+p	MT20	3.0	4.0	Edge	
3	TMVW+p	MT20	4.0	5.0	1.75	2.00
5	BMV1+p	MT20	2.0	4.0		
6	BMWWW-t	MT20	3.0	6.0		
7	BMV+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
8	366	0	366	0	0	1-8	1-8
5	473	0	473	0	0	3-0	3-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
8	257	179 / 0	0 / 0	0 / 0	0 / 0	77 / 0	0 / 0
5	330	244 / 0	0 / 0	0 / 0	0 / 0	85 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 8, 5

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	-210 / 0	-77.3	-77.3	0.12 (1)	6.25	6- 2	0 / 67	0.02 (4)
2- 3	-210 / 0	-77.3	-77.3	0.19 (1)	6.25	1- 6	0 / 185	0.04 (1)
3- 4	0 / 38	-77.3	-77.3	0.11 (1)	10.00	6- 3	0 / 160	0.04 (1)
7- 1	-377 / 0	0.0	0.0	0.05 (1)	7.81			
5- 3	-463 / 0	0.0	0.0	0.05 (1)	7.81			
8- 7	0 / 0	-94.8	-94.8	0.39 (1)	10.00			
7- 6	0 / 0	-17.5	-17.5	0.39 (1)	10.00			
6- 5	0 / 0	-17.5	-17.5	0.12 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.26")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.26")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.19 (2-3:1) , BC=0.39 (7-8:1) , WB=0.04 (1-6:1) , SSI=0.29 (7-8:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.35 (6) (INPUT = 0.90)
JSI METAL= 0.09 (5) (INPUT = 1.00)

RECEIVED
TOWN OF MILTON
MAR 29, 2017
JUNIPER 9
BUILDING DIVISION

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

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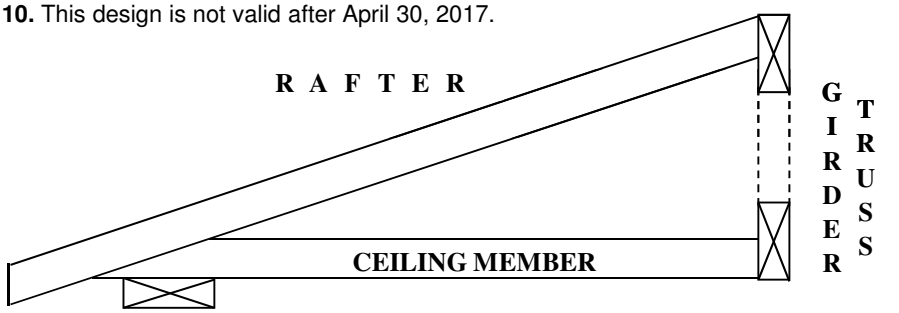
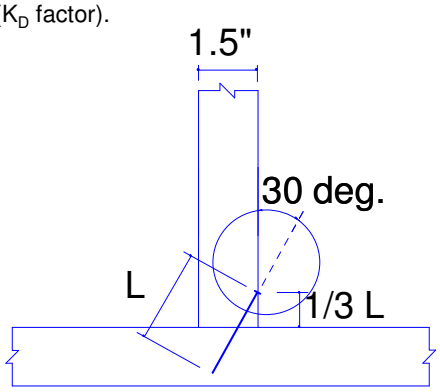
BEARING ANCHORAGE BY TOE-NAILS FOR LATERAL CAPACITY B37579H1

NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL LATERAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	132	147
	3.25	0.144	132	147
	3.50	0.160	159	177
COMMON SPIRAL	3.00	0.122	97	108
	3.25	0.122	97	108
	3.50	0.152	145	162

NOTES:

1. Rafter and ceiling members may be anchored to top and bottom chords of girder truss by toe-nailing rafter and ceiling members to girder chords provided the reaction does not exceed the lateral capacities in the table. Hangers (specified by others) are required for reactions higher than the maximum toe-nail capacity. Reactions are based on factored loads.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-09, section 10.9.4.1.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in tables below.
5. Nail values in table are based on the following relative lumber densities: G = 0.42 (SPF), G = 0.49 (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30º to the grain of the member (See next page for nailing on bearing plate).
7. For loads due to **wind** the nail lateral capacity in this table may be multiplied by 1.15 (K_D factor).
8. Lumber must be dry (< 19% moisture content) at the time of nail installation.
9. Nail values in this table comply with CSA O86-09, section 10.9.4
10. This design is not valid after April 30, 2017.





TOE-NAIL INSTALLATION

Nail type	Common wire	Common spiral	Common wire	Common spiral
Nail dia. (in)	0.160	0.152	0.144	0.122
	(3.5" nail)		(3" and 3.25" nail)	
LUMBER SIZE	**MAXIMUM NUMBER OF TOE-NAILS**			
2X4 SPF	2	2	3	3
2X4 D. Fir	2	2	2	2
2X6 SPF	4	4	4	5
2X6 D. Fir	3	3	3	4

MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

PEO
Certificate No. 10889485

April 24, 2015

BEARING ANCHORAGE BY TOE-NAILS FOR WIND LOADING B37579H2

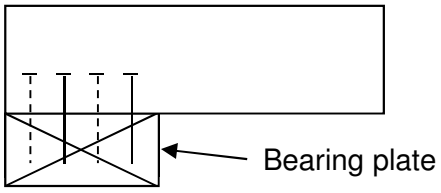
NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL WITHDRAWAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	30	42
	3.25	0.144	32	45
	3.50	0.160	38	52
COMMON SPIRAL	3.00	0.122	26	36
	3.25	0.122	28	40
	3.50	0.152	36	50

Note: If using truss with D. Fir lumber and S-P-F bearing plate, use values in table for S-P-F.

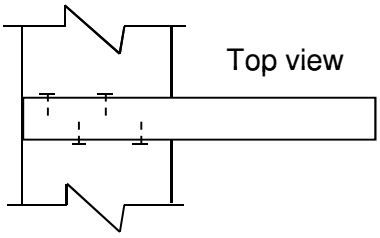
NOTES:

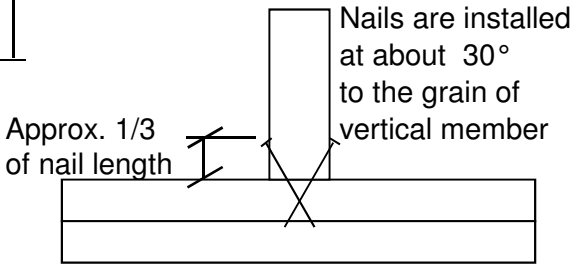
1. Truss chord, rafter, or ceiling members may be anchored to bearing plate by toe-nails, provided that the actual factored uplift force due to **wind** or **earthquake** load does not exceed the withdrawal capacities in the table. Hangers (specified by others) are required for uplift forces that are higher than the maximum toe-nail withdrawal capacity.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-09, section 10.9.5.2.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in table above.
5. Nail values in table are based on the following relative lumber densities: G = 0.42(SPF), G = 0.49(D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See drawing on detail B37579H1).
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-09, section 10.9.5
9. This design is not valid after April 30, 2017

Toe-nailing on 2x6 Bearing Plate



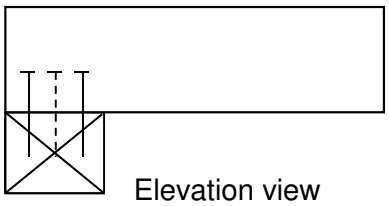
Elevation view



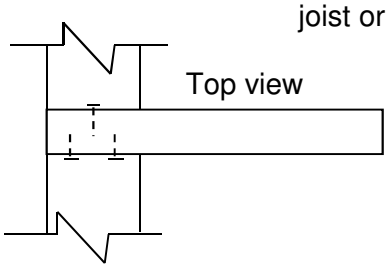


Toe-nailing viewed from end of joist or truss

Toe-nailing on 2x4 Bearing Plate



Elevation view



MiTek Canada Inc
100 Industrial Rd.
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MAR 29, 2017
JUNIPER 9
BUILDING DIVISION

PEO
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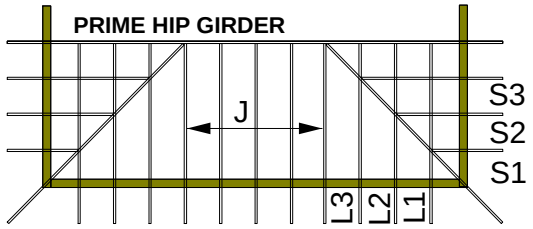
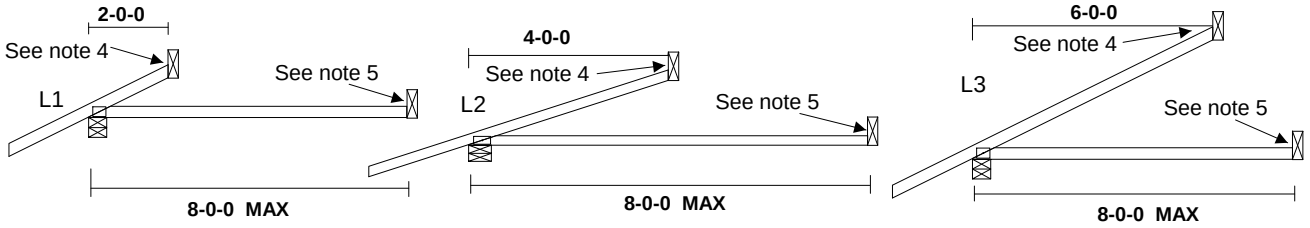
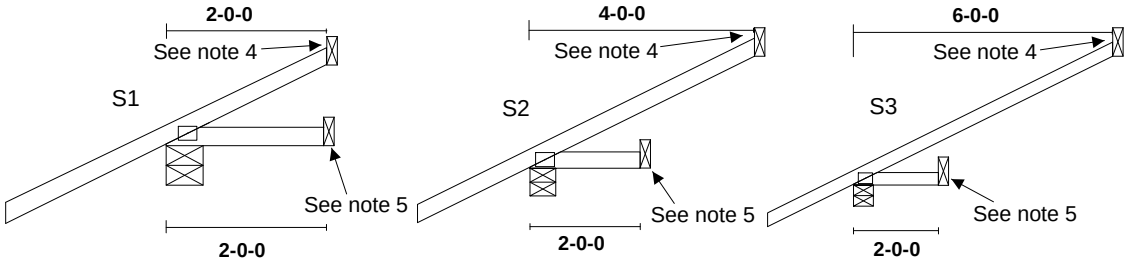
April 24, 2015

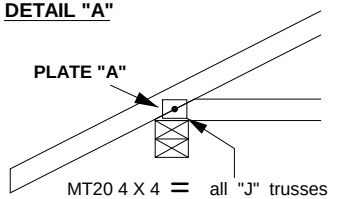
JOB NAME	JOB DESC.	DRAWING NO.
DETAILS	HIP END FRAMING LEGGOONRIDGE-JUNIPER 9 EL 2	B37579J





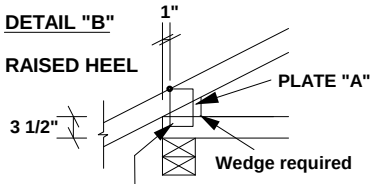


DETAIL "A"



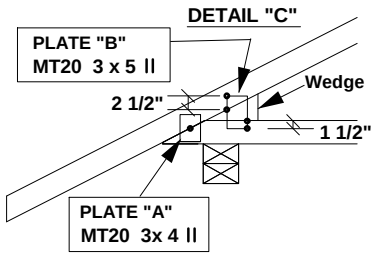
MT20 4 X 4 = all "J" trusses
MT20 3 X 4 = all other trusses

DETAIL "B"



MT20 4 X 8 II all 11/12 & 12/12 pitch trusses
MT20 4 X 7 II all other trusses

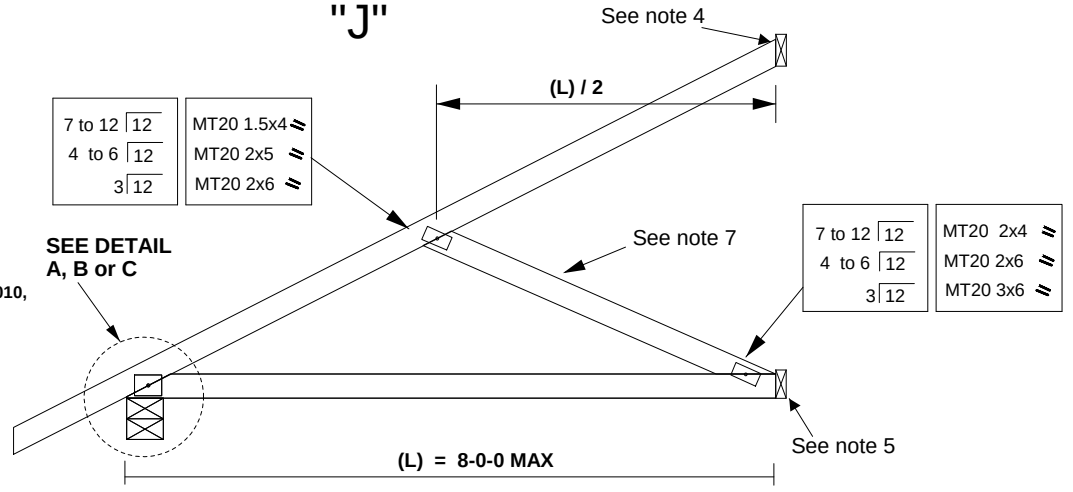
DETAIL "C"



CANTILEVER DETAIL "C"

SLOPE	MAX CANTILEVER	PLATE "B"	WEDGE SIZE
3/12	17"	3 X 5	2 X 3
4/12	14"	3 X 5	2 X 3
5/12	12"	3 X 5	2 X 4
6/12	10"	3 X 5	2 X 4
7/12	9"	3 X 5	2 X 6
8/12	8.5"	3 X 5	2 X 6
9/12	8"	3 X 5	2 X 6
10/12	7.5"	3 X 5	2 X 6

"J"



NOTES:

1. ALL LUMBER SHALL BE 2x4 SPF OR D. Fir No. 2 DRY OR BETTER.
2. THIS TRUSS IS DESIGNED FOR HOUSING AND SMALL BUILDING REQUIREMENTS OF PART 9 NBCC 2010, WHERE GROUND SNOW LOAD IS 60.0 PSF OR LESS AND RAIN LOAD DOES NOT EXCEED 12.53 PSF; TOP CHORD DEAD LOAD IS 6 PSF OR LESS; BC LIVE LOAD IS 0 PSF AND BC DEAD LOAD IS 7 PSF.
3. HIP RAFTER DESIGN SHALL CONFORM TO SECTION 9.23.14.6 OF NBCC 2010.
4. FASTEN HIGH END OF RAFTERS USING MITEK CANADA INC. "BEARING ANCHORAGE BY TOE-NAILS FOR LATERAL CAPACITY" STANDARD DETAIL B37579H1.
5. FASTEN RIGHT END OF CEILING USING MITEK CANADA INC. "BEARING ANCHORAGE BY TOE-NAILS FOR LATERAL CAPACITY" STANDARD DETAIL B37579H1.
6. OVERHANG LENGTH SHALL NOT EXCEED 2 FT.
7. WHEN SETBACK IS 6 FT OR LESS, DIAGONAL WEB MAY BE OMITTED AND HIGH END OF TOP CHORD SHALL BE CONNECTED AS PER NOTE 4.
8. ALL PLATES SPECIFIED ARE PRESSED INTO BOTH FACES OF THE TRUSS.
9. MITEK REFERENCE PAGE MII-7473C FORMS AN INTEGRAL PART OF THIS DETAIL.
10. THIS DETAIL IS NOT VALID AFTER APRIL 30, 2017

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MiTek Canada, Inc.
100 Industrial Rd.
Bradford, Ontario, L3Z 3G7

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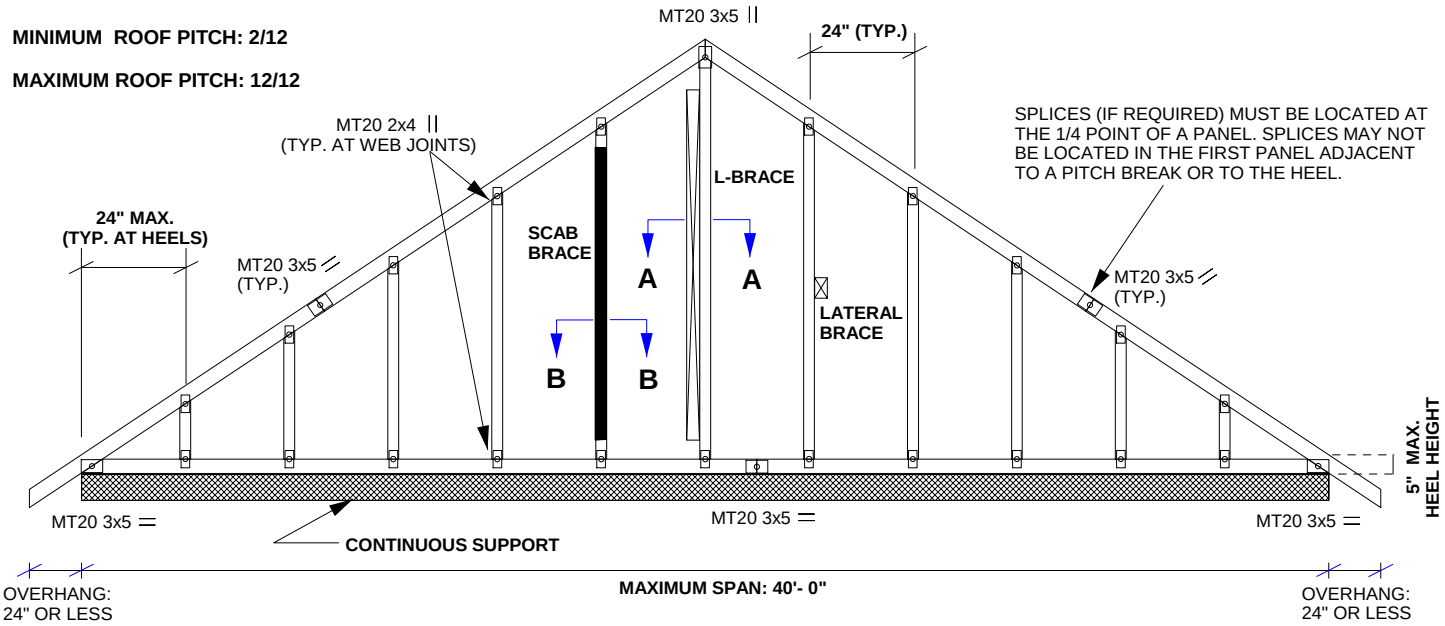
Certificate No. 10889485

LICENSED PROFESSIONAL ENGINEER
C. Cordogiannis
90553736
PROVINCE OF ONTARIO

April 24, 2015

STANDARD GABLE END DETAIL

LECCO RIDGE-JUNIPER 9 EL 2 **DRAWING NO.:** ENG JOB: P7001179 **B37579K**



LUMBER

TOP CHORD:	**2 X 4 No. 2 DRY SPF or D- Fir**
BOTTOM CHORD:	**2 X 3 or 2 X 4 No. 2 DRY SPF or D- Fir**
GABLE WEB:	**2 X 3 or 2 X 4 No. 2 DRY SPF or D- Fir**

PLATES

JOINT	PLATES
HEELS	MT20 3 X 5
PEAK	MT20 3 X 5
TC SPLICES	MT20 3 X 5
BC SPLICES	MT20 3 X 5
WEB JOINTS	MT20 2 X 4

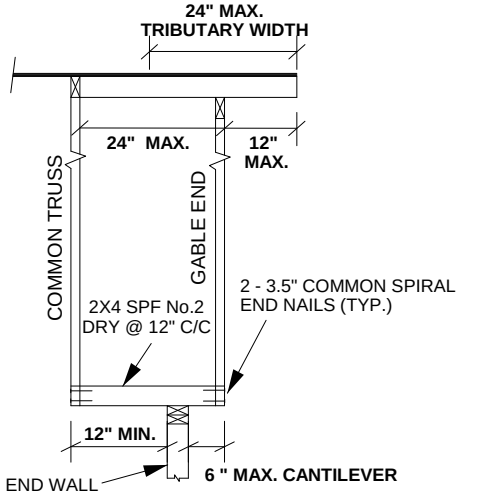
DESIGN CRITERIA

TOP CHORD LL = 60.0 PSF OR LESS
TOP CHORD DL = 6.0 PSF OR LESS
BOTTOM CHORD LL = 0 PSF
BOTTOM CHORD DL = 7.0 PSF

TOTAL LOAD = 73.0 PSF OR LESS

CANTILEVER DETAIL

Note: Gable end may be cantilevered up to 6 inches past end wall as shown. Gable end to be continuously supported by 2x4 SPF No.2 (DRY) members at 12" o.c. along the bottom chord. Roof design loads shall not exceed the loading shown above.

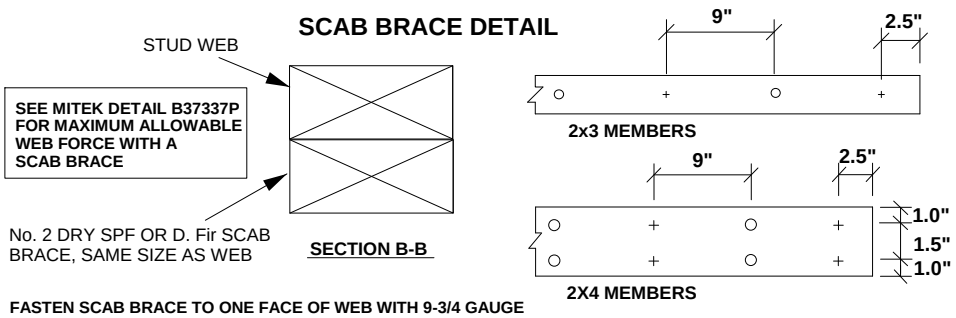


BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10 FT. OR RIGID CEILING DIRECTLY APPLIED.
WEBS MUST BE LATERALLY BRACED, SCAB BRACED OR L-BRACED AS INDICATED IN TABLE BELOW:

WEB LENGTH (L)	SCAB BRACE	L-BRACE	LATERAL BRACE
L < 6 FT.	NOT REQUIRED	NOT REQUIRED	NOT REQUIRED
6 FT. < L < 12 FT.	REQUIRED	2x4 L-BRACE	1 LATERAL AT 1/2 LENGTH OF WEB

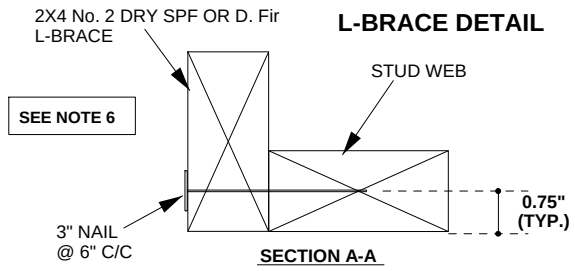
SCAB BRACE DETAIL



FASTEN SCAB BRACE TO ONE FACE OF WEB WITH 9-3/4 GAUGE 0.122" X 3" COMMON SPIRAL NAILS SPACED @ 9" C/C (MAX) WITH 2.5" MINIMUM END DISTANCE. SCAB BRACE MUST COVER 90% OF WEB LENGTH. DRIVE NAILS ALTERNATELY FROM FRONT AND BACK FACES.

\+ NAIL FROM FRONT FACE
o NAIL FROM BACK FACE

L-BRACE DETAIL



FASTEN L-BRACE TO NARROW EDGE OF WEB WITH ONE ROW OF 9-3/4 GAUGE 0.122" X 3" COMMON SPIRAL NAILS SPACED AT 6" C/C WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

NOTES:

1. **Gable studs are spaced at 24" C/C (max.) with a max. length of 12 ft.**
2. **All plates specified are MiTek MT20, centered at each joint, and pressed into both faces of truss.**
3. **Truss spacing is 24" C/C, maximum.**
4. **Gable truss is designed for continuous support. Bearing material must be of the same species as chord member and of grade No. 2 or better.**
5. **This truss requires rigid sheathing attached to exposed face.**
6. **2x3 or 2x4 T-braces shown for gable webs in the MiTek engineering drawings may be replaced by a 2x4 L-brace as shown above.**
7. **This truss is designed for residential or small building requirements, conforming to Part 9, NBCC 2010.**
8. **This detail is not valid after April 30, 2017.**

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April 24, 2015

MiTek Canada, Inc.
100 Industrial Rd.
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