

BUILDING: REVIEWED

SCOTT SHERRIFFS APR 19, 2017
PLANS EXAMINER DATE

Neither the issuance of a permit nor carrying out of inspections by the Town of Milton relives the owner fron full responsibility for compliance with the provisions of the Ontario Building Code Act and the Ontario Building Code, both as amended, as well as other applicable statutes and regulations of the Province on Ontario, By-laws of the Region of Halton and Town of Milton

RECEIVED TOWN OF MILTON MAR 29, 2017 17-4978 BUILDING DIVISION





UNDERSIDE OF SOFFIT FOR PORCHES AT 8' 6" A.F.F.

GARAGE WALL PLATES AT 9' 1 2/16" A.F.F.

PARTY WALLS TO BE FRAMED ON SITE A.P.P.

# HANGER LEGEND:

▼ LUS24 ● HGUS26 ■ LJS26DS × HGUS26-2



# CONVENTIONAL FRAMING BY OTHERS

SIZE AND LOCATION OF CONVENTIONAL FRAMING IS APPROXIMATE. ALL AREAS MAY NOT BE SHOWN. REFER TO ARCHITECTURAL PLANS FOR DETAILS.

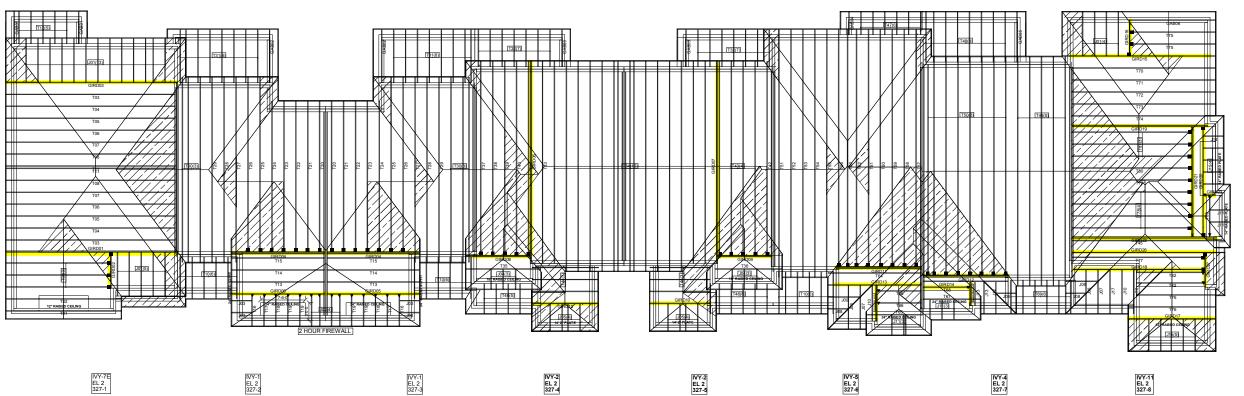
ALL CONVENTIONAL FRAMING TO CONFORM WITH PART 9 OF THE O.B.C
ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4 S.P.F. @ 24"O/C WITH A 2x4 VERTICAL POST TO THE TRUSS UNDERNEATH EACH CROSS POINT. VERTICAL POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END POINT AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

Model: **BLOCK 327**Customer: GREENPARK

Project: LECCO RIDGE

Location: MILTON

Date: 3/9/2017 Drawn by: BB



# <u>ENGINEERING NOTE PAGE (ENP-1)</u> PLEASE READ PRIOR TO INSTALLATION

# RESPONSIBILITIES

THIS DESIGN IS FOR AN INDIVIDUAL BUILDING COMPONENT AND HAS BEEN BASED ON INFORMATION PROVIDED BY THE DESIGN OFFICE OF KOTT LUMBER. THE UNDERSIGNED ENGINEER DISCLAIMS ANY RESPONSIBILITY FOR DAMAGES AS A RESULT OF FAULTY OR INCORRECT INFORMATION, SPECIFICATION AND/OR DESIGNS FURNISHED TO THE ENGINEER. THE UNDERSIGNED ENGINEER IS ONLY RESPONSIBLE FOR THE STRUCTURAL INTEGRITY OF THIS BUILDING COMPONENT FOR THE CONDITIONS AND LOADS SHOWN ON THIS DRAWING. THE STRUCTURAL INTEGRITY OF THE BUILDING AND THE VERIFICATION OF THE DIMENSIONS AND THE DESIGN LOADS USED ARE THE RESPONSIBILITY OF THE BUILDING DESIGNER.

TRUSSES ARE DESIGNED IN CONFORMANCE WITH THE RELEVANT SECTIONS OF THE NATIONAL BUILDING CODE OF CANADA OR THE CANADIAN CODE FOR FARM BUILDINGS, WHICHEVER APPLIES TO THE BUILDING TYPE INDICATED ON THE DRAWING

IT IS THE RESPONSIBILITY OF KOTT LUMBER TO ENSURE THAT TRUSSES ARE MANUFACTURED IN CONFORMANCE WITH THESE DESIGNS AND WITH THE SPECIFICATIONS OUTLINED BELOW. THE UNDERSIGNED ENGINEER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

### **USE AND OCCUPANCY**

The building is of the type indicated on the drawing

### **LOADING**

- The truss loading intensity and distribution as well as load transfer mechanism is that indicated on the drawing
- No buildings, trees, parapets or other projections higher than the roof for which the trusses are used are
  located within a distance less than ten (10) times the difference in height, or five metres (16 ft)
  whichever is greater, unless the drawing indicates that the snow drifting has been taken into account

## HANDLING, INSTALLATION AND BRACING

- The trusses must be handled and installed by a qualified professional as per the supplied document titled *Information for Truss Installers* and the BCSI-B1 and BCSI-B3 Summary Sheets
- The compression chords are laterally braced by continuous rigid diaphragm sheathing or as specified on the drawing
- Temporary and permanent bracing must be installed as indicated on the truss drawing and according to the BCSI-B1 and BCSI-B3 Summary Sheets. Bracing for the lateral stability of the truss is to be provided by the building designer
- It is recommended that a Professional Engineer's advice be obtained for the bracing of trusses spanning more than 12.37m (40'-7")

# **SUPPORTS**

- The trusses are to be supported at the bearing points indicated and anchored to the supports where considered necessary by the designer of the overall structure
- Bearing sizes shown are the minimum required to prevent crushing of the truss members and do not necessarily take into account stability of the overall building structure
- Elevation of bearings must be carefully checked and shimmed to alignment for solid bearings
- Adequate wood truss bearing is the responsibility of the building designer.

### **DIMENSIONS**

Geometry of the truss and dimensions indicated on the drawing are identical to those of the installed truss.

RECEIVED
TOWN OF MILTON

MAR 29, 2017 17-4978

BUILDING DIVISION

JOB PESC TR-GREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY DRWG NO. 2 TW0317-048 GAB01

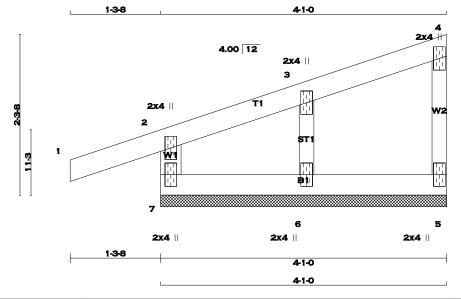
Kott Lumber Uxbridge, Stouffville, ON, TW

Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:16 2017 Page 1 ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-9QMzFmlj38Z6IXOTqjotqPtTzEV2GG5U\_IPjIGzcJKD

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TW0317-048

SCALE = 1:16.4



LUMBER N. L. G. A. RULES CHORDS SIZE DESCR LUMBER 2 2x4 DRY No.2 No.2 SPF SPF DRY 2x3 DRY No 2 SPF No.2 ALL WEBS 2x3 ALL GABLE WEBS DRY No.2 SPF DRY No.2 SPF DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

#### PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Υ						
2	TMV+p	MT20	2.0	4.0							
3	TMW+w	MT20	2.0	4.0							
4	TMV+p	MT20	2.0	4.0							
5	BMV1+p	MT20	2.0	4.0							
6	BMW1+w	MT20	2.0	4.0							
7	BMV1+p	MT20	2.0	4.0							

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY **BUILDING DESIGNER BEARINGS** 

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

PROVIDE ANCHORAGE AT ALL BEARING JOINTS FOR 150 LBS FACTORED UPLIFT.

PROVIDE FOR 155 LBS FACTORED HORIZONTAL REACTION AT JOINT 7

#### HORIZONTAL REACTIONS

	1ST LCASE	MAX./N	IIN. COMPO				
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
7		0/0	0/0	0/0	111/0	0/0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

TOTAL LOAD CASES: (11)

			WE	BS		
ED FACTORED				MAX. FACT	ORED	
CE VERT. LOAD LC	1 MAX	MAX.	MEMB.	FORCE	MAX	
(PLF)	CSI (LC)	<b>UNBRAC</b>		(LBS)	CSI (LC)	
FROM TO		<b>LENGTH</b>	FR-TO			
0.0 0.0	0.07 (6)	7.81	6-3	-170 / 145	0.03 (7)	
-77.3 -77.3	0.10(1)	10.00				
-77.3 -77.3	0.04(1)	6.25				
-77.3 -77.3	0.04(1)	6.25				
0.0 0.0	0.06 (5)	7.81				
-175 -175	0.02 (6)	6 25				
	CE VERT. LOAD LC (PLF) FROM TO 2 0.0 0.0 -77.3 -77.3 -77.3 -77.3 -77.3 -77.3 0.0 0.0	CE VERT. LOAD LC1 MAX (PLF) CSI (LC) FROM TO 0.0 0.0 0.07 (6) -77.3 -77.3 0.04 (1) -77.3 -77.3 0.04 (1) 0.0 0.0 0.06 (5) -17.5 -17.5 0.02 (6)	CE VERT. LOAD LC1 MAX MAX.  (PLF) CSI (LC) UNBRAC FROM TO LENGTH  2 0.0 0.0 0.07 (6) 7.81 -77.3 -77.3 0.10 (1) 10.00 -77.3 -77.3 0.04 (1) 6.25 -77.3 -77.3 0.04 (1) 6.25 -77.3 -17.5 0.02 (6) 6.25	ED FACTORED C VERT. LOAD LC1 MAX MAX. MEMB. (PLF) CSI (LC) UNBRAC EROM TO LENGTH FR-TO 2 0.0 0.07 (6) 7.81 6-3 7.73 -77.3 0.04 (1) 6.25 7.73 -77.3 0.04 (1) 6.25 0.0 0.0 0.06 (5) 7.81 1.15 1.75 -17.5 0.02 (6) 6.25	CE VERT. LOAD LC1 MAX MAX. MEMB. FORCE (LBS)  FROM TO LENGTH FR-TO  0.0 0.0 0.07 (6) 7.81 6-3 -170 / 145  -77.3 -77.3 0.04 (1) 6.25  -77.3 -77.3 0.04 (1) 6.25  0.0 0.0 0.0 0.06 (5) 7.81  -17.5 -17.5 0.02 (6) 6.25	ED FACTORED MAX. FACTORED CE VERT. LOAD LC1 MAX MAX. MEMB. FORCE MAX (PLF) CSI (LC) UNBRAC (LBS) CSI (LC) FROM TO LENGTH FR-TO 2 0.0 0.0 0.07 (6) 7.81 6-3 -170 / 145 0.03 (7) -77.3 -77.3 0.10 (1) 10.00 -77.3 -77.3 0.04 (1) 6.25 -77.3 -77.3 0.04 (1) 6.25 -77.3 -77.3 0.04 (1) 6.25 -77.3 -77.3 0.04 (1) 6.25 -77.3 -77.3 0.04 (1) 6.25

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (\$3.0) PSP AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.

### **DESIGN CRITERIA**

SPECIFIED LOADS:

LL = DL = LL = DL = AD = CH. 3.0 PSF 7.0 PSF TOTAL LOAD 33.3 PSF

#### SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

TOTAL WEIGHT = 2 X 14 = 28 lb

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014

- CSA 086-09 TPIC 2011

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.10 (1-2:1) , BC=0.02 (6-7:6) , WB=0.03 (3-6:7) , SSI=0.08 (2-7:6)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (3) (INPUT = 0.90 ) JSI METAL= 0.08 (7) (INPUT = 1.00 )



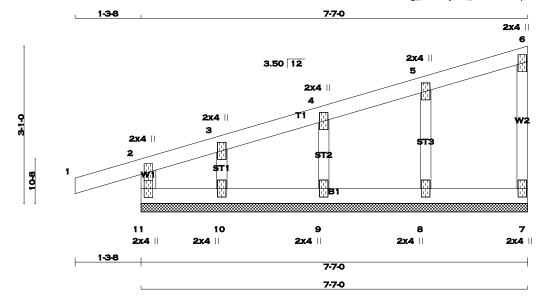


READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

JOB PESC TR-GREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY PLY DRWG NO. Page 3 of 159 2 TW0317-048 TW0317-048 GAB02

Kott Lumber Uxbridge, Stouffville, ON, TW

Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:17 2017 Page 1 ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-ddwLT6JLqRizNhzfNRJ6MdQejerx?iLdDP9HqizcJKC



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR 11 -2 2x4 2x4 DRY No.2 No.2 SPF SPF DRY 2x3 DRY No 2 SPF No.2 ALL WEBS 2x3 ALL GABLE WEBS DRY No.2 SPF DRY No.2 SPF DRY: SEASONED LUMBER

GABLE STUDS SPACED AT 2-0-0 OC.

BMV1+p

#### PLATES (table is in inches) W LEN Y TMV+p MT20 2.0 4.0 TMW+w MT20 2.0 4.0 TMV+p MT20 2.0 4.0 BMV1+p MT20 4.0 8, 9, 10 8 BM\ 11 BM\ BMW1+w

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TI 20 PLATES IS ALLOWED.

2.0

MT20

4.0

2.25 1.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER **BEARINGS** 

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

PROVIDE ANCHORAGE AT ALL BEARING JOINTS FOR 150 LBS FACTORED UPLIFT.

PROVIDE FOR 203 LBS FACTORED HORIZONTAL REACTION AT JOINT 11

	1ST LCASE	MAX./N	MAX./MIN. COMPONENT REACTIONS							
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL			
11		0/0	0/0	0/0	145 / 0	0/0	0 /0			

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (11)

	RDS					WE		
		FACTO					MAX. FACTO	
MEMB.	FORCE	VERT. LO	AD LC1	I MAX	MAX.	MEMB.	FORCE	MAX
	(LBS)	(PL	.F) (	CSI (LC)	<b>UNBRAG</b>	0	(LBS)	CSI (LC)
FR-TO	, ,	FROM			LENGTH	FR-TO	, ,	` ,
11-2	-194 / 88	0.0	0.0	0.06(7)	7.81	8- 5	-168 / 132	0.03(1)
1- 2	0 / 14	-77.3	-77.3	0.10(1)	10.00	9- 4	-160 / 140	0.03 (5)
2-3	-124 / 0	-77.3	-77.3	0.07(1)	6.25	10-3	-101 / 129	0.03 (7)
3-4	-95 / 0	-77.3	-77.3	0.04(1)	6.25			
4- 5	-70 / 22	-77.3	-77.3	0.04(1)	6.25			
5-6	-38 / 45	-77.3	-77.3	0.04 (5)	6.25			
7-6	-66 / 57	0.0	0.0	0.08 (5)	7.81			
11-10	-42 / 48	-17.5	-17.5	0.05 (6)	6.25			
10-9	-37 / 53	-17.5	-17.5	0.02 (5)	6.25			
9-8	-35 / 56	-17.5	-17.5	0.02 (11	) 6.25			
8- 7	-33 / 59	-17.5		0.03 (5)	6.25			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.

# TOTAL WEIGHT = 2 X 25 = 50 lb **DESIGN CRITERIA**

SPECIFIED LOADS: CH.

LL = DL = LL = DL = AD = PSF 3.0 PSF 7.0 PSF TOTAL LOAD 33.3 PSF

#### SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

SCALE = 1:22.6

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014 - CSA 086-09

TPIC 2011

DESIGN ASSUMPTIONS -OVERHANG NOT TO BE ALTERED OR CUT

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.10 (1-2:1) , BC=0.05 (10-11:6) , WB=0.03 (4-9:5) , SSI=0.11 (2-11:6)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.16 (11) (INPUT = 0.90 ) JSI METAL= 0.10 (11) (INPUT = 1.00 )





READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

JOB PESC TR-GREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY DRWG NO. 2 TW0317-048 GAB03

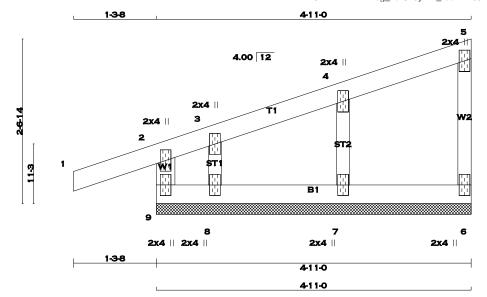
Kott Lumber Uxbridge, Stouffville, ON, TW

Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:17 2017 Page 1 ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-ddwLT6JLqRizNhzfNRJ6MdQeier1?iKdDP9HqizcJKC

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SCALE = 1:18.0



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR 2 2x4 DRY No.2 No.2 SPF SPF 2x4 DRY 2x3 DRY No 2 SPF No.2 ALL WEBS 2x3 ALL GABLE WEBS DRY No.2 SPF DRY No.2 SPF DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

# PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Υ
2	TMV+p	MT20	2.0	4.0	
3	TMW+w	MT20	2.0	4.0	
4	TMW+w	MT20	2.0	4.0	
5	TMV+p	MT20	2.0	4.0	
6	BMV1+p	MT20	2.0	4.0	
7	BMW1+w	MT20	2.0	4.0	
8	BMW1+w	MT20	2.0	4.0	
9	BMV1+p	MT20	2.0	4.0	
	•				

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

### **BEARINGS**

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

PROVIDE ANCHORAGE AT ALL BEARING JOINTS FOR 150 LBS FACTORED UPLIFT.

PROVIDE FOR 175 LBS FACTORED HORIZONTAL REACTION AT JOINT 9

#### HORIZONTAL REACTIONS

	1ST LCASE	MAX./I	MIN. COMPO				
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
9		0/0	0/0	0/0	125 / 0	0/0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (11)

					WE		
FACTORED	FACTOR	RED				MAX. FACTO	DRED
FORCE	VERT. LO	AD LC	1 MAX	MAX.	MEMB.	FORCE	MAX
(LBS)	(PL	.F) (	CSI (LC)	UNBRA(	2	(LBS)	CSI (LC)
	FROM	TO		LENGTH	FR-TO		
-193 / 78	0.0	0.0	0.06 (7)	7.81	7- 4	-181 / 147	0.03 (7)
0 / 16	-77.3	-77.3	0.10(1)	10.00	8-3	-46 / 104	0.02 (7)
-101 / 0	-77.3	-77.3	0.08 (1)	6.25			
-72 / 7	-77.3	-77.3	0.05 (1)	6.25			
-36 / 35	-77.3	-77.3	0.05 (1)	6.25			
-64 / 54	0.0	0.0	0.06 (5)	7.81			
-34 / 44							
-30 / 48	-17.5	-17.5	0.02 (5)	6.25			
	(LBS) -193 / 78 0 / 16 -101 / 0 -72 / 7 -36 / 35 -64 / 54 -38 / 39 -34 / 44	FACTORED FACTOR FORCE (LBS) (PROM 1-193 / 78	FACTORED FORCE VERT. LOAD LC (LBS) (PLF) (FROM TO 0.193 / 78 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.	FACTORED FORCE VERT. LOAD LC1 MAX (LBS) (PLF) CSI (LC) FROM TO 0.0 0.06 (7) 0/16 -77.3 -77.3 0.08 (1) -72/7 -77.3 -77.3 0.05 (1) -36/35 -77.3 -77.3 0.05 (1) -64/54 0.0 0.0 0.06 (5) -38/39 -17.5 -17.5 0.04 (6) -34/44 -17.5 -17.5 0.02 (11	FACTORED FORCE VERT. LOAD LC1 MAX MAX. (LBS) (PLF) CSI (LC) UNBRAY FROM TO LENGTH 0.0 0.0 0.06 (7) 7.81 0.10 0.0 0.07 0.07 0.0 0.00 0.0 0.0 0.0 0.0	FACTORED FORCE (URT. LOAD LC1 MAX MAX. MEMB. (LBS) (UF) CSI (LC) UNBRAC ENGTH FR-TO (URT)	FACTORED FORCE VERT. LOAD LC1 MAX MAX. MEMB. FORCE (LBS) (PLF) CSI (LC) UNBRAC (LBS)  -193 / 78

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT (40-0-0) FIN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK
COEFFICIENTS, CPCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL
WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON
(OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY

#### **DESIGN CRITERIA**

SPECIFIED LOADS: CH.

LL = DL = LL = DL = AD = PSF 3.0 PSF 7.0 PSF TOTAL LOAD 33.3 PSF

#### SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

TOTAL WEIGHT = 2 X 17 = 34 lb

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014

- CSA 086-09 TPIC 2011

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.10 (1-2:1) . BC=0.04 (8-9:6) . WB=0.03 (4-7:7), SSI=0.09 (2-9:6)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (4) (INPUT = 0.90 ) JSI METAL= 0.09 (9) (INPUT = 1.00 )





READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

JOB NAME TRUSS NAME QUANTITY PLY

GAB04

JOB PESC TR-GREENPARK-LECCO RIDGE-BLOCK 327 TRUSS DESC.

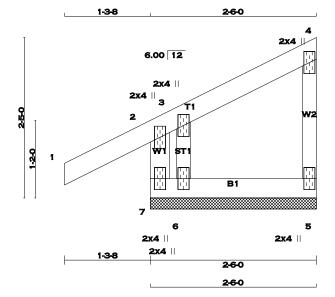
DRWG NO.

Page 5 of 159 TW0317-048

Kott Lumber Uxbridge, Stouffville, ON, TW Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:17 2017 Page 1 ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-ddwLT6JLqRizNhzfNRJ6MdQeeert?iKdDP9HqizcJKC

SCALE = 1:17.3

TOTAL WEIGHT = 11 lb



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR 2 2x4 DRY No.2 No.2 SPF SPF 2x4 DRY 2x3 DRY No 2 SPF No.2 ALL WEBS 2x3 ALL GABLE WEBS DRY No.2 SPF DRY No.2 SPF DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

#### PLATES (table is in inches)

TW0317-048

JT	TYPE	PLATES	W	LEN	Υ					
2	TMV+p	MT20	2.0	4.0						
3	TMW+w	MT20	2.0	4.0						
4	TMV+p	MT20	2.0	4.0						
5	BMV1+p	MT20	2.0	4.0						
6	BMW1+w	MT20	2.0	4.0						
7	BMV1+p	MT20	2.0	4.0						

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER **BEARINGS** 

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

PROVIDE ANCHORAGE AT ALL BEARING JOINTS FOR 150 LBS FACTORED UPLIFT EXCEPT 6:196 LBS.

PROVIDE FOR 154 LBS FACTORED HORIZONTAL REACTION AT JOINT 7

### HORIZONTAL REACTIONS

	1ST LCASE	MAX./N	IIN. COMPO				
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
7		0/0	0/0	0/0	110 / -49	0/0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

<u>BRACING</u> TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (12)

CHC	ORDS					WE	BS	
MAX.	FACTORED	FACTO	RED				MAX. FACTO	RED
MEMB.	FORCE	VERT. LC	AD LC	1 MAX	MAX.	MEMB.	FORCE	MAX
	(LBS)	(Pl	_F) (	CSI (LC)	UNBRAC	)	(LBS)	CSI (LC)
FR-TO		FROM	TO		LENGTH	FR-TO		
7-2	-220 / 45	0.0	0.0	0.05 (7)	7.81	6-3	-49 / 146	0.03 (6)
1- 2	0 / 23	-77.3	-77.3	0.10(1)	10.00			
2-3	-99 / 26	-77.3	-77.3	0.08 (1)	6.25			
3- 4	-39 / 33	-77.3	-77.3	0.04(1)	6.25			
5- 4	-72 / 58	0.0	0.0	0.06 (7)	7.81			
7-6	-36 / 43	-17.5	-17.5	0.05 (6)	6.25			
6- 5	-27 / 44	-17.5	-17.5	0.02 (5)	6.25			

## CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT { 40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

### **DESIGN CRITERIA**

SPECIFIED LOADS:

LL = DL = LL = DL = AD = CH. PSF 3.0 PSF PSF 7.0 PSF TOTAL LOAD 33.3 PSF

#### SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014 - CSA 086-09

DESIGN ASSUMPTIONS

TPIC 2011

-OVERHANG NOT TO BE ALTERED OR CUT

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.10 (1-2:1) . BC=0.05 (6-7:6) . WB=0.03 (3-6:6) , SSI=0.08 (2-3:12)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (3) (INPUT = 0.90 ) JSI METAL= 0.08 (7) (INPUT = 1.00 )





READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

JOB PESC TR-GREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY PLY DRWG NO. TRUSS DESC TW0317-048 GAB05

Kott Lumber Uxbridge, Stouffville, ON, TW

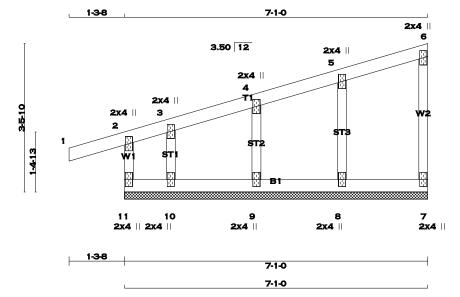
Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:17 2017 Page 1 ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-ddwLT6JLqRizNhzfNRJ6MdQc5eq3?iJdDP9HqizcJKC

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TW0317-048

SCALE = 1:26.9

TOTAL WEIGHT = 25 lb



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR 11 -2 2x3 DRY No.2 No.2 SPF SPF 2x4 DRY 2x3 DRY No 2 SPF No.2 ALL WEBS 2x3 ALL GABLE WEBS DRY No.2 SPF DRY No.2 SPF DRY: SEASONED LUMBER

GABLE STUDS SPACED AT 2-0-0 OC.

#### PLATES (table is in inches) W LEN Y TMV+p MT20 2.0 4.0 TMW+w MT20 2.0 4.0 TMV+p MT20 2.0 BMV1+p MT20 4.0 8, 9, 10 8 BM\ 11 BM\ BMW1+w BMV1+p MT20 2.0 4.0

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TI 20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY **BUILDING DESIGNER** 

### **BEARINGS**

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

PROVIDE ANCHORAGE AT ALL BEARING JOINTS FOR 150 LBS FACTORED UPLIFT EXCEPT 10:231 LBS.

PROVIDE FOR 228 LBS FACTORED HORIZONTAL REACTION AT JOINT 11

### HORIZONTAL REACTIONS

	1ST LCASE	MAX./N	IIN. COMPO				
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
11		0/0	0/0	0/0	163 / -3	0/0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

<u>BRACING</u> TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (11)

CH	ORDS					WE	BS	
MAX	. FACTORED	FACTO	RED				MAX. FACTO	RED
MEMB.	FORCE	VERT. LO	AD LC	1 MAX	MAX.	MEMB.	FORCE	MAX
	(LBS)	(PL	.F) (	CSI (LC)	<b>UNBRAC</b>	)	(LBS)	CSI (LC)
FR-TO		FROM	TO		LENGTH	FR-TO		
11-2	-209 / 68	0.0	0.0	0.20 (6)	7.81	8- 5	-169 / 124	0.03(1)
1- 2	0 / 14	-77.3	-77.3	0.10(1)	10.00	9- 4	-162 / 143	0.03 (5)
2- 3	-135 / 0	-77.3	-77.3	0.09(4)	6.25	10-3	-54 / 150	0.03(6)
3- 4	-94 / 6	-77.3	-77.3	0.04(1)	6.25			
4- 5	-70 / 31	-77.3	-77.3	0.04(1)	6.25			
5- 6	-38 / 54	-77.3	-77.3	0.06 (5)	6.25			
7- 6	-66 / 61	0.0	0.0	0.11 (5)	7.81			
11-10	-50 / 58	-17.5	-17.5	0.10 (6)	6.25			
10- 9	-46 / 61	-17.5	-17.5	0.02(5)	6.25			
9-8	-44 / 64	-17.5	-17.5	0.02(5)	6.25			
8- 7	-42 / 66	-17.5	-17.5	0.04 (5)	6.25			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.

### **DESIGN CRITERIA**

SPECIFIED LOADS:

LL = DL = LL = DL = AD = CH. PSF 3.0 PSF PSF 7.0 PSF TOTAL LOAD 33.3 PSF

#### SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014 - CSA 086-09

TPIC 2011

DESIGN ASSUMPTIONS -OVERHANG NOT TO BE ALTERED OR CUT

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.20 (2-11:6) , BC=0.10 (10-11:6) , WB=0.03 (3-10:6) , SSI=0.17 (2-11:6)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (11) (INPUT = 0.90 ) JSI METAL= 0.12 (11) (INPUT = 1.00 )





READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

JOB NAME TRUSS NAME QUANTITY

TW0317-048

GAB06

1-3-8

JOB PESC TR-GREENPARK-LECCO RIDGE-BLOCK 327

3x4 II

DRWG NO.

Page 7 of 159 TW0317-048

SCALE = 1:37.9

TOTAL WEIGHT = 53 lb

Kott Lumber Uxbridge, Stouffville, ON, TW

Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:18 2017 Page 1 ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-5pTjgSKzblqq?rXrx8qLvqyoL2AJk8knS3vqN8zcJKB

6 8.00 12 5 4 T1 ST4 9 10 ST2 ST2 1-3-8 <sub>1|9 18</sub> 13 17 16 13<del>0</del>0 1300

LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR 19 -1 -6 -11 -2 2x4 DRY No.2 No.2 SPF SPF 2x4 DRY 10 2x4 DRY No 2 SPF 10 2x3 No.2 19 -11 2x4 DRY No.2 SPF ALL WEBS 2x3 DRY No.2 SPF ALL GABLE WEBS 2x3 DRY
DRY: SEASONED LUMBER. No.2 SPF

GABLE STUDS SPACED AT 2-0-0 OC.

PL/	ATES	(table	is i	n inches	)
.IT	TYPF		PL	ATES	· w

JT	TYPE	PLATES	W	LEN	Υ	Χ
2	TMV+p	MT20	2.0	4.0		
3, 4	, 5, 7, 8, 9					
3	TMW+w	MT20	2.0	4.0		
6	TTW+p	MT20	3.0	4.0	2.25	1.50
10	TMV+p	MT20	2.0	4.0		
11	BMV1+p	MT20	2.0	4.0		
12,	13, 14, 15, 16	5, 17, 18				
12	BMW1+w	MT20	2.0	4.0		
19	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

#### DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER **BEARINGS**

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

6-6-0

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

PROVIDE ANCHORAGE AT ALL BEARING JOINTS FOR 150 LBS FACTORED UPLIFT EXCEPT 19:288 LBS,11:167 LBS,16:154 LBS,17:158 LBS,18:373 LBS,14:151 LBS,13:164 LBS.12:278 LBS

PROVIDE FOR 256 LBS FACTORED HORIZONTAL REACTION AT JOINT 19

### HORIZONTAL REACTIONS

	1ST LCASE	MAX./N	IIN. COMPO	NENT REACTION	SNC		
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
19		0/0	0/0	0/0	183 / -174	0/0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (11)

CHC	RDS	WEBS							
MAX.	FACTORED	FACTOR	RED				MAX. FACTO	RED	
MEMB.	FORCE	VERT. LO.	AD LC1	I MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PL	F) (	CSI (LC)	<b>UNBRAC</b>	)	(LBS)	CSI (LC)	
FR-TO		FROM	TO		LENGTH	FR-TO			
19- 2	-258 / 138	0.0	0.0	0.10 (6)	7.81	15-6	-168 / 5	0.08 (1)	
1- 2	0 / 29	-77.3	-77.3	0.10(1)	10.00	16- 5	-188 / 178	0.05(3)	
2-3	-200 / 172	-77.3	-77.3	0.09(4)	6.25	17- 4	-184 / 191	0.04 (7)	
3- 4	-95 / 126	-77.3	-77.3	0.04(8)	6.25	18- 3	-98 / 202	0.04 (6)	
4- 5	-37 / 183	-77.3	-77.3	0.05 (7)	6.25	14- 7	-186 / 175	0.05 (4)	
5-6	-33 / 238	-77.3	-77.3	0.08(8)	6.25	13-8	-187 / 193	0.04 (8)	
6- 7	-26 / 221	-77.3	-77.3	0.08(8)	6.25	12- 9	-160 / 215	0.04 (8)	
	-17 / 147	-77.3	-77.3	0.06(8)	6.25				
8- 9	-28 / 79			0.04 (8)					
9-10	-117 / 89			0.04 (6)					
11-10	-136 / 94	0.0	0.0	0.10 (5)	7.81				
19-18	-85 / 115			0.10 (6)					
18-17	-73 / 109	-17.5	-17.5	0.03 (5)	6.25				
17-16	-69 / 109	-17.5	-17.5	0.02 (5)	6.25				
16-15	-67 / 109	-17.5	-17.5	0.02(5)	6.25				
15-14	-67 / 109	-17.5	-17.5	0.02(5)	6.25				
14-13	-67 / 107	-17.5	-17.5	0.02(5)	6.25				
13-12	-67 / 104	-17.5	-17.5	0.02(5)	6.25				
12-11	-63 / 93	-17.5	-17.5	0.06 (5)	6.25				

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CPCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM FAVE



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

**RECEIVED** TOWN OF MILTON MAR 29, 2017 17-4978 **BUILDING DIVISION** 

### **DESIGN CRITERIA**

#### SPECIFIED I DADS

SPECIFIED LUADS.									
TOP	CH.	LL	=	23.3	PSF				
		DL	=	3.0	PSF				
BOT	CH.	LL	=	0.0	PSF				
		DL	=	7.0	PSF				
TOTA	1 10	۸D	_	22.2	DOE				

#### SPACING = 24.0

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014 - CSA 086-09

TPIC 2011

# DESIGN ASSUMPTIONS -OVERHANG NOT TO BE ALTERED OR CUT

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.10 (1-2:1) , BC=0.10 (18-19:6) , WB=0.08 (6-15:1) , SSI=0.13 (18-19:6)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (6) (INPUT = 0.90 ) JSI METAL= 0.16 (6) (INPUT = 1.00 )

JOB PESC TR-GREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY DRWG NO. Page 8 of 159 2 TW0317-048 TW0317-048 **GIRD01** Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:18 2017 Page 1 Kott Lumber Uxbridge, Stouffville, ON, TW ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-5pTjgSKzblqq?rXrx8qLvqylz26Kk4znS3vqN8zcJKB 1.3-8 4-1-12 19-1-7 3-7-12 SCALE = 1:45.6 6x8 = **3x8** = 4x5 = 4x4 || 4x4 = 8x8 // 3 5 6 8 4 **T2** 8.00 12 5x8 = 5x6 / 10 **B**1 П B2 S S 陆 15 18 17 16 14 13 12 19 **5x6** = 3x4 II 4x6 5x5 = 4x5 = 6x8 5x5 4x6 3x4 II 1-3-8 26-6-0 5-8 1/12 27-1-4 TOTAL WEIGHT = 2 X 128 = 257 lb LUMBER N. L. G. A. RULES CHORDS SIZE DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER **DESIGN CRITERIA** LUMBER DESCR BEARINGS FACTORED 3 DRY No.2 SPF MAXIMUM FACTORED INPLIT REQRD \*\*\* SPECIAL LOADS ANALYSIS \*\*\*
GEOMETRY AND/OR BASIC LOADS CHANGED No.2 GROSS REACTION GROSS REACTION BRG LIPLIFT IN-SX 9 2x4 DRY No 2 SPF VFRT HOR7 DOWN HOR7 IN-SX BY USER 10 2x4 No.2 19 11 2072 196 -1106 LOADS WERE DERIVED FROM USER INPUT 19 -2 2x4 DRY No.2 SPF 2716 3035 -1461 1-12 1-12 NO FURTHER MODIFICATIONS WERE MADE 10 No.2 PROVIDE ANCHORAGE AT BEARING JOINT 19 FOR 1106 LBS FACTORED UPLIFT SPECIFIED LOADS: 19 -15 2x6 No.2 LL = DL = LL = 15 -No.2 SPF PROVIDE ANCHORAGE AT BEARING JOINT 11 FOR 1461 LBS\_FACTORED\_UPLIFT TOP CH. 23.3 PSF PSF PSF 3.0 NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES ALL WEBS DRY SPF BOT CH. 2x3 No.2 DL **EXCEPT** PSF TOTAL LOAD 33.3 PROVIDE FOR 196 LBS FACTORED HORIZONTAL REACTION AT JOINT 19 DRY: SEASONED LUMBER. SPACING = 24.0 IN. C/C DESIGN CONSISTS OF <u>2</u> TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS: UNFACTORED REACTIONS MAX./MIN. COMPONENT REACTIONS
SNOW LIVE PERM.LIVE WIND 1ST LCASE COMBINED DEAD LOADING IN FLAT SECTION BASED ON A SLOPE SOIL 497 / -1060 797 / -1415 0/0 1451 1030 / 0 0/0 0/0 421 / 0 OF 6.00/12 SURFACE LOAD(PLF) CHORDS #ROWS SPACING (IN) GIRDER TYPE: CPrimeHip TOP CHORDS : (0.122"X3") SPIRAL NAILS HORIZONTAL REACTIONS LEFT SETBACK = 4-1-12 RIGHT SETBACK = 3-10-0 0/0 0/0 142 / -128 0/0 0 /0 1-3 3-7 19 0/0 SIDF(44.8) END SETBACK = 6-0-0 END WALL WIDTH = 5-8 CORNER FRAMING TYPE: CONVENTIONAL 12 12 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 19, 11 SIDE(44.8) 9-10 12 SIDE(0.0) END JACK TYPE: CONVENTIONAL APPLIED TO FRONT SIDE 19- 2 TOP TOP TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.96 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY 11-10 BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS ADDT'L LOADS BASED ON 55 % OF GSL TOP LOADS APPLIED TO FIRST 10-10-0 OF SPAN SIDE(8.7) MEASURED FROM THE RIGHT. 15-11 12 WEBS: (0.122"X3") SPIRAL NAILS ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED. \*\*\* NON STANDARD GIRDER \*\*\* 6 LOADING TOTAL LOAD CASES: (11) ADDT'L USER-DEFINED LOADS APPLIED TO NAILS TO BE DRIVEN FROM ONE SIDE ONLY. ALL LOAD CASES. THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010 GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS. CHORDS WEBS MAX. FACTORED MAX. FACTORED FACTORED FORCE MEMB FORCE VERT. LOAD LC1 MAX MAX. MEMB. TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR (PLF) FROM TO CSI (LC) UNBRAC THIS DESIGN COMPLIES WITH: FR-TO LENGTH FR-TO 1- 2 2- 3 3- 4 4- 5 0.06 (1) THE LOAD TO BE TRANSFERRED TO EACH PLY 0/29 -77.3 -77.3 -77.3 0.06 (1) -77.3 0.23 (7) 10.00 18- 3 3-17 -407 / 267 PART 9 OF OBC 2012, BCBC 2012, ABC 2014 0.05 (3) -2598 / 1292 -1131 / 2230 - CSA 086-09 5.51 0.27 (8) SIDE - PLE SHOWN IS THE FOLIVALENT LIDEAPPLIED -3790 / 1957 -77.3 -77 3 0 21 (7) 4 80 17-4 -1473 / 832 0.18 (3) - TPIC 2011 TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. -77.3 0.26 (7) -810 / 1636 4983 / 2519 4.27 4-16 0.20 (8) (55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. 5-6 0.12 (3) -5707 / 2854 -77.3 -77.3 0.27 (8) 4.05 16-5 -999 / 578 REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP. 6- 7 7- 8 -5707 / 2854 -5707 / 2854 -145.8 -145.8 -145.8 -145.8 0.32 (8) 3.96 3.96 5-14 14- 6 -487 / 996 -429 / 323 0.12 (8) 0.05 (3) RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD 8-9 -4610 / 2355 -145 8 -145 8 0 28 (8) 4 36 14-8 -772 / 1616 0.18(1) ALLOWABLE DEFL.(LL)= L/360 (0.90") CALCULATED VERT. DEFL.(LL) = L/999 (0.12") ALLOWABLE DEFL.(TL)= L/360 (0.90") CALCULATED VERT. DEFL.(TL) = L/999 (0.19") 9-10 19-2 -3339 / 1669 -2239 / 1127 13- 8 13- 9 -77.3 0.23 (8) 5.05 -1858 / 1060 0.22 (3) PLATES (table is in inches) 0.0 0.0 0.11(1) 7.50 -1362 / 2740 0.30(1)-841 / 542 0.0 TYPE TMVW-t W LEN Y 11-10 -2982 / 1497 0.0 0.15 (1) 12-9 0.10 (3) 6.73 1.75 3.00 -953 / 2189 5.0 6.0 2-18 0.25(1)19-18 -177 / 169 -17.5 -17.5 0.02 (11) 6.25 12-10 -1357 / 2943 0.32(1) 18-17 -1005 / 2163 -17.5 0.14 (1) 6.25 CSI: TC=0.32 (6-8:8) , BC=0.36 (14-16:1) TI WISE THE 17-16 -1801 / 3792 -17.5 -17.5 0.25 (1) 6.25 WB=0.32 (10-12:1), SSI=0.13 (8-9:3) 16-15 15-14 -2362 / 4984 -2362 / 4984 -17.5 -17.5 -17.5 0.36 (1) -17.5 0.36 (1) 6.25 6.25 DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 14-13 -2164 / 4612 -33.0 -33.0 0.33 (1) 6.25 COMP=1.00 SHEAR=1.00 TENS= 1.00 13-12 -1238 / 2751 -33.0 0.17 COMPANION LIVE LOAD FACTOR = 0.50 12-11 -12/26-33.0 -33.0 0.03 (11) 6.25 FACTORED CONCENTRATED LOADS (LBS) JT. LOC LC1 MAX-MAX+ FACE DIR TYPF TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE VERT TOTAL 100083566 14 ONT VERT TOTAL TRUSS MANUFACTURING PLANT. NAIL VALUES RESSURE OF (20) PSF A PLATE GRIP(DRY) SHEAR SECTION WINI ERENCE VELOCITY P (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN E GRADE AND USING (40-0 C) IN-SX TELLER LINE HEIGHT ABOVE GRADE AND USING COEFFICIENTS, CPCg, BASED ON THE (MAIN WIND FORCE RESIS WCE OF ONTAR INCTOYSTEMONTENILATION
MAY BE LOCATED ON WIN {OPE 618 354 1667 822 2284 1656 READ ALL NOTES ON THIS PAGE AND ON THE EAST (NADA FT-1209SX240WARY ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE PLATE PLACEMENT TOL. = 0.250 inches March 10, 2017 IS AN INTEGRAL PART OF THIS DRAWING AS IT 17-4978

CONTAINS SPECIFICATIONS AND CRITERIA USED

IN THE DESIGN OF THIS COMPONENT.

PLATE ROTATION TOL. = 5.0 Deg.

**BUILDING DIVISION** 

JOB DESC TR-GREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY PLY DRWG NO. TRUSS DESC. 2 TW0317-048 GIRD01

Kott Lumber Uxbridge, Stouffville, ON, TW

Version 8.100 S Feb. 9.2017 MTek Industries, Inc. Fri Mar 10.14:20:19.2017 Page 2 ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-Z?15toLbM3yhc?62VsLaR2VwjSSZTXDwgjeNvbzcJKA

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JSI GRIP= 0.56 (12) (INPUT = 0.90 ) JSI METAL= 0.42 (15) (INPUT = 1.00 )

PLATES (table is in inches)
JT TYPE PLATES W
3 TTWW-m MT20 6.0
4 TMWW+t MT20 4.0
6 TMW+w MT20 2.0
7 TS-t MT20 3.0
8 TMW+t MT20 3.0
9 TTWW+m MT20 8.0
10 TMVW-p MT20 3.0
11 BMV1+p MT20 3.0
12 BMWW-t MT20 4.0
13 BMWW-t MT20 4.0
13 BMWW-t MT20 6.0
14 BMWW-t MT20 6.0
15 BS-t MT20 5.0
16 BMWW-t MT20 5.0
17 BMWW-t MT20 5.0
18 BS-t MT20 5.0
18 BMWW-t MT20 5.0
19 BMWW-t MT20 5.0
19 BMWW-t MT20 5.0
11 BMWW-t MT20 5.0
12 BMWW-t MT20 5.0
13 BMWW-t MT20 5.0
14 BMWW-t MT20 5.0
15 BS-t MT20 5.0
16 BMWW-t MT20 4.0
17 BMWW-t MT20 4.0
18 BMWW-t MT20 4.0 8.0 2.00 3.00
4.0 1.50 1.50
4.0 8.0 Edge 3.00
8.0 1.50 Edge 4.0 2.25 1.50
6.0 1.75 1.50
5.0 1.75 1.50
6.0 1.75 2.50
6.0 1.75 2.50
6.0 1.75 2.50
6.0 1.75 2.50
6.0 1.75 2.50 1.75 2.25 2.25 1.75 1.75 2.25

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

6.0 4.0

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 290.7 lbs FACTORED DOWN AND 201.8
lbs FACTORED UP AT 23-3-4 ON TOP CHORD,
AND 1441.5 lbs FACTORED DOWN AND 665.9 lbs
FACTORED UP AT 16-3-4 ON BOTTOM CHORD.
DESIGN FOR UNSPECIFIED CONNECTION(S) IS
DELECATED TO THE BILL DIME DESIGNED. DELEGATED TO THE BUILDING DESIGNER.

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

JOB NAME

Kott Lumber Uxbridge, Stouffville, ON, TW

TW0317-048

TRUSS NAME **GIRD02**  QUANTITY PLY POBPES GREENPARK-LECCO RIDGE-BLOCK 327 TRUSS DESC

DRWG NO.

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SCALE = 1:30.4

Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:19 2017 Page 1 ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-Z?15toLbM3yhc?62VsLaR2Vy5SSzTYMwgjeNvbzcJKA

600 2x4 || 3 6.00 12 4x4 / 3x6 / 1.20 W2 В 5 4x4 = <sup>4</sup> 4x6 Ⅱ 2x4 || 530 5-8 600

TOTAL WEIGHT = 29 lb

LUMBER				
N. L. G. A. R	ULES			
CHORDS	SIZE		LUMBER	DESCR
1 - 3	2x4	DRY	No.2	SPF
4 - 3	2x4	DRY	No.2	SPF
6 - 1	2x4	DRY	No.2	SPF
6 - 4	2x6	DRY	No.2	SPF
· .				
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER

### PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Υ	X
1	TMVW-t	MT20	3.0	6.0		
2	TMWW-t	MT20	4.0	4.0	1.75	1.25
3	TMV+p	MT20	2.0	4.0		
4	BMVW1-t	MT20	4.0	4.0	1.75	1.75
5	BMWW+t	MT20	4.0	6.0	3.25	2.00
6	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY **BUILDING DESIGNER** 

DEA	KINGS						
	FACTO	RED	MAXIMU	IM FACT	ORED	INPUT	REQRD
	GROSS RI	EACTION	GROSS	REACTIO	N	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
4	1263	0	1456	0	-537	HANGER E	BY OTHERS
						MIN. SEAT	SIZE: 3-8
6	1263	0	1455	242	-471	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 4 FOR 537 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 6 FOR 471 LBS FACTORED UPLIFT

ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES. SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 242 LBS FACTORED HORIZONTAL REACTION AT JOINT 6

### LINEACTORED REACTIONS

	1ST LCASE	MAX./	MIN. COMPO	NENT REACTION	ONS			
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
4	886	620 / 0	0/0	0/0	483 / -555	266 / 0	0/0	
6	886	620 / 0	0/0	0/0	481 / -508	266 / 0	0/0	
HORIZONTAL REACTIONS								
6		0/0	0/0	0/0	173 / -70	0/0	0 /0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 6

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.66 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (11)

CHORDS						W E	BS		
MAX	K. FACTORED	FACTORED				MAX. FACTORED			
MEMB.	FORCE	VERT. LC	DAD LC	1 MAX	MAX.	MEMB	. FORCE	MAX	
	(LBS)	(PI	_F) (	CSI (LC)	UNBRA(	2	(LBS)	CSI (LC)	
FR-TO		FROM	TO		LENGTH	FR-TO	)		
1- 2	-1253 / 408	-77.3	-77.3	0.17 (7)	5.66	5- 2	-312 / 1117	0.24(3)	
2-3	-72 / 71	-77.3	-77.3	0.11 (7)	6.25	2- 4	-1406 / 571	0.31(3)	
4- 3	-102 / 78	0.0	0.0	0.10(7)	7.81	1- 5	-323 / 1167	0.25 (4)	
6- 1	-950 / 335	0.0	0.0	0.09(1)	7.81				
6- 5	-226 / 91	-343.5	-343.5	0.21 (3)	6.25				
5- 4	-432 / 1122	-343.5	-343.5	0.33(3)	6.25				

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.

#### **DESIGN CRITERIA**

SPEC	IFIED	LOAL	DS:		
TOP	CH.	LL	=	23.3	PSF
		DL	=	3.0	PSF
	~				

PSF PSF LL = 0.0 DL = 7.0 AD = 33.3 BOT CH. TOTAL LOAD PSF

### SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder START DISTANCE = 0-0 START SPAN CARRIED = 16-2-8 END DISTANCE = 6-0-0 END SPAN CARRIED = 16-2-8 END WALL WIDTH = 5-8 APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF

### THIS DESIGN COMPLIES WITH:

PART 9, NBCC 2010

- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09
- TPIC 2011

(55 % OF 27 2 P.S.F. G.S.I. PLUS 8 4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20\*) CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02\*) ALLOWABLE DEFL.(TL)= L/360 (0.20\*) CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03\*)

CSI: TC=0.17 (1-2:7) , BC=0.33 (4-5:3) , WB=0.31 (2-4:3) , SSI=0.40 (5-6:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

### NAIL VALUES

PLATE GRIP(DRY) SHEAR (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN MAX MIN

MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (5) (INPUT = 0.90) JSI METAL= 0.42 (2) (INPUT = 1.00)





READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

JOB PESC TR-GREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY DRWG NO Page 11 of 159 TW0317-048 TW0317-048 **GIRD03** Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:20 2017 Page Kott Lumber Uxbridge, Stouffville, ON, TW ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-1CbT58LE7M4YE8hE3Zsp\_F2wprhwCtp3vNOxR1zcJK9 1-3-8 41-12 41-12 SCALE = 1:46.3 4x4 = 6x8 = **4**x**4** = 2x4 3x6 6x8 = 5 6 **T2** 8.00 12 E E 4x16 = 4x16 = 8 13 16 15 14 12 10 4x8 = 3x10 = 4x6 = 3x8 3x4 || 4x6 = 3x10 = 1·3·8 5·6 26-6-0 27.5-0 TOTAL WEIGHT = 110 lb LUMBER N. L. G. A. RULES CHORDS SIZE DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER **DESIGN CRITERIA** LUMBER DESCR BEARINGS FACTORED 3 DRY No.2 SPF MAXIMUM FACTORED INPLIT REQRD SPECIFIED LOADS: LL = DL = LL = DL = AD = No.2 GROSS REACTION GROSS REACTION CH. LIPLIFT IN-SX 3.0 2x4 DRY No 2 VFRT HOR7 DOWN HOR7 IN-SX PSF 2x4 No.2 195 -1393 5-8 17 -2x6 DRY No.2 SPF 10 2408 -1331 5-8 5-8 7.0 **PSF** 10 -9 No.2 TOTAL LOAD 33.3 PROVIDE ANCHORAGE AT BEARING JOINT 17 FOR 1393 LBS FACTORED UPLIFT 13 No.2 13 -10 No.2 SPACING = 24.0 IN. C/C NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES ALL WEBS DRY SPF 2x3 No.2 **EXCEPT** LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12 PROVIDE FOR 195 LBS FACTORED HORIZONTAL REACTION AT JOINT 17 DRY: SEASONED LUMBER. GIRDER TYPE: CPrimeHip UNFACTORED REACTIONS SIDE SETBACK = 4-1-12 1ST LCASE COMBINED MAX./MIN. COMPONENT REACTIONS
SNOW LIVE PERM.LIVE WIND END SETBACK = 6-0-0 END WALL WIDTH = 5-8 DEAD SOIL 888 / -1330 0/0 PLATES (table is in inches)
JT TYPE PLATES 1241 / 0 0/0 0/0 522 / 0 CORNER FRAMING TYPE: CONVENTIONAL 10 END JACK TYPE: CONVENTIONAL W LEN Y TMVW-p MT20 4.0 16.0 Edge 5.50 APPLIED TO FRONT SIDE TTWW-m MT20 8.0 4.0 2.00 2.75 2.00 1.75 HORIZONTAL REACTIONS 17 --- 0/0 - ADDT'L LOADS BASED ON 55 % OF GSL 0/0 0/0 139 / -131 0/0 TMWW-t 0 /0 2.0 3.0 4.0 THIS TRUSS IS DESIGNED FOR RESIDENTIAL TMW+w MT20 4 0 6.0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 17, 10 OR SMALL BUILDING REQUIREMENTS OF TMWW-t 4.0 MT20 2.00 1.75 PART 9, NBCC 2010 6.0 4.0 3.0 ΓTWW-m 8.0 2.00 2.75 MT20 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.14 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 4.46 FT. OR RIGID CEILING DIRECTLY MT20 16.0 Edge 5.50 THIS DESIGN COMPLIES WITH: q-WVMT 10 BMV1+r MT20 40 PART 9 OF OBC 2012 BCBC 2012 ABC 2014 BMWW-t 3.0 4.0 10.0 1.50 3.75 CSA 086-09 12 BMWW-t MT20 6.0 1.50 2.50 BS-4.0 8.0 MT20 ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED. BMWWW-t (55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. MT20 6.0 10.0 1.50 2.50 LOADING TOTAL LOAD CASES: (11) RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED BMWW-t MT20 4.0 BMWW-t 1.50 3.75 ROOF LIVE LOAD 17 BMV1+p MT20 3.0 4.0 ALLOWABLE DEFL.(LL)= L/360 (0.91\*) CALCULATED VERT. DEFL.(LL)= L/ 999 (0.26\*) ALLOWABLE DEFL.(TL)= L/360 (0.91\*) CALCULATED VERT. DEFL.(TL)= L/786 (0.42\*) CHORDS MAX. FACTORED WEBS Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD. MAX. FACTORED FACTORED MEMB FORCE VERT. LOAD LC1 MAX MAX. MEMB. (PLF) FROM TO CSI (LC) UNBRAC A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH FR-TO LENGTH FR-TO TEE-LOK TL20 PLATES IS ALLOWED. 1-2 2-3 0/29 -77.3 -77.3 -77.3 -77.3 0.11 (1) 0.52 (7) 10.00 3.52 16- 3 3-15 -519 / 432 CSI: TC=1.00 (5-7:3), BC=0.80 (12-14:1), WB=0.81 (8-12:7), SSI=0.37 (7-8:3) -3292 / 1666 -1264 / 2418 0.81(8)3- 4 4- 5 5- 6 6- 7 7- 8 -4624 / 2417 -145.8 -145.8 0.93 (3) 2 39 15-4 -1358 / 901 0.35 (3) 2.14 -5164 / 2658 -145.8 -145.8 1.00 (3) -359 / 701 HANGERS NOTES DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 314.4 lbs FACTORED DOWN AND 218.2 14- 5 14- 7 12- 7 COMP=1.00 SHEAR=1.00 TENS= 1.00 -5164 / 2658 -145.8 -145.8 1.00(3) -744 / 543 0.19(3)-5164 / 2658 -4607 / 2410 -145.8 -145.8 -145.8 -145.8 1.00 (3) 0.92 (3) 2.14 2.40 -364 / 720 -1372 / 904 0.23 (7) 0.35 (3) COMPANION LIVE LOAD FACTOR = 0.50 lbs FACTORED UP AT 23-3-4, AND 314.4 lbs FACTORED DOWN AND 218.2 lbs FACTORED UP AT 4-1-12 ON TOP CHORD. DESIGN FOR 8- 9 17- 2 -3270 / 1654 -77.3 -77 3 0 52 (8) 3.56 12-8 -1268 / 2439 0.81 (7) -521 / 437 -1278 / 2794 6.26 TRUSS PLATE MANUFACTURER IS NOT 10-9 -2670 / 1370 0.0 0.0 0.17(1) 6.39 2-16 0.61(1) UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER. RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. -1299 / 2804 0.61 (1) 17-16 -174 / 173 -33.0 -33.0 0.17 (11) 6.25 16-15 -1308 / 2737 -33.0 -33.0 -33.0 0.51 (1) -33.0 0.80 (1) 5.57 15-14 -2257 / 4626

> 218 4-1-12 -241 FRONT VERT TOTAL FRONT VFRT TOTAL WIN

-33.0 0.80 (1)

-33.0 0.80 (1) -33.0 0.51 (1)

-33.0 0.17 (11)

MAX+

ERENCE VELOCITY PRESSURE OF ( 9.0) PSF AT /E GRADE AND USING EXTERNAL PEAK I WIND FORCE RESIS ING SYSTEM INTERNAL TEGORY 2), BUILDING MAY BRUDGATER VISION OM R HT AE HE {M (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT EASTOU/WHITONESMIALVIAON

FACE

4.50

4.50 5.71

10.00

DIR

TYPE

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

-33.0

-33.0 -33.0

-33.0

MAX-

FACTORED CONCENTRATED LOADS (LBS)

LC1

TI WISE THE

100083566

WCE OF ONTAR

March 10, 2017

14-13 13-12 12-11

11-10

.IT

-2216 / 4609

-2216 / 4609 -1226 / 2703

-10/21

MAR 29, 2017 17-4978 **BUILDING DIVISION**  PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (15) (INPUT = 0.90)

JSI METAL= 0.93 (13) (INPUT = 1.00)

618 354 1667 822 2284 1656

JOB NAME TRUSS NAME

Kott Lumber Uxbridge, Stouffville, ON, TW

TW0317-048

LUMBER N. L. G. A. RULES CHORDS SIZE

ALL WEBS 2x3

2x4

2x4

2x4

2x4

2x6

DRY: SEASONED LUMBER

DRY

DRY

DRY

DRY

DRY

DESIGN CONSISTS OF  $\underline{\mathbf{3}}$  TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS

SURFACE

12

12

BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS

WEBS : (0.122"X3") SPIRAL NAILS

SPACING (IN) TOP CHORDS : (0.122"X3") SPIRAL NAILS

STAGGER NAILS BY HALF THE SURFACE SPACING IN

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED

TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE

W

3.0

3.0 4.0 4.0 4.0

2.0

4.0

3.0 6.0

4.0 5.0

TI WISE THE

100083566

WCE OF ONTAR

March 10, 2017

LEN Y

1.50 1.50

1.50 1.00

2.50 1.50

2.75 1.75

4.0

40

4.0

THE LOAD TO BE TRANSFERRED TO EACH PLY.

4

. 10 -

10 -

EXCEPT

FOLLOWS:

4-6

6- 7

CHORDS #ROWS

ADJACENT PLIES.

SIDE OR ON THE TOP.

TMWW-t

TMWW+t

BMVW1-t

BMWW+t

BMWW+1

10 BMV1+p

TMV+n

PLATES (table is in inches)

MT20

MT20 MT20

MT20

MT20

MT20

MT20

MT20

LUMBER

No.2 No.2

No 2

No.2

No.2

No.2

DESCR

SPF SPF

SPF

SPF

SPF

LOAD(PLF)

TOP

TOP

TOP

TOP

SIDE(471.5)

QUANTITY 2 **GIRD04** 

PLY

3

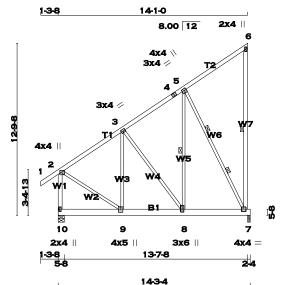
JOB PESC TR-GREENPARK-LECCO RIDGE-BLOCK 327 TRUSS DESC

DRWG NO

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SCALE = 1:85.6

Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:20 2017 Page 1 ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-1CbT58LE7M4YE8hE3Zsp\_F25jrofCru3vNOxR1zcJK9



TOTAL WEIGHT = 6 X 94 = 563 lb DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER **DESIGN CRITERIA** MAXIMUM FACTORED INPLIT REQRD SPECIFIED LOADS:

BEARINGS FACTORED GROSS REACTION GROSS REACTION BRG HORZ UPLIFT IN-SX JT. VFRT HOR7 DOWN IN-SX 4320 -1832 10 4725 5490 761 -1689 5-8 5-8

PROVIDE ANCHORAGE AT BEARING JOINT 7 FOR 1832 LBS FACTORED PROVIDE ANCHORAGE AT BEARING JOINT 10 FOR 1689 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES
SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 761 LBS FACTORED HORIZONTAL REACTION AT JOINT 10

### UNFACTORED REACTIONS

	151 LUASI	<u> </u>	WIN. COMPON	IENT REACT	IONS				
JT	COMBINE	D SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
7	3032	2121 / 0	0/0	0/0	1917 / -1894	911/0	0/0		
10	3314	2332 / 0	0/0	0/0	1911 / -1838	982 / 0	0/0		
HOR	IZONTAL R	EACTIONS				911/0 0/0			
10		0/0	0/0	0/0	544 / -377	0/0	0 /0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 7, 10

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.85 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 6-7. DBS = 20-0-0 . CBF = 19 LBS. - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 5-8. DBS = 6-0-0 . CBF = 73 LBS. 2 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/3 LENGTH OF 5-7. DBS = 4-0-0. CBF = 90 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) TO EACH PLY USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

## **LOADING**

TOTAL LOAD CASES: (11)

СН	ORDS					W E	BS		
MAX	. FACTORED	FACTO	RED				MAX. FACTO	RED	
MEMB.	FORCE	VERT. LC	DAD LC	1 MAX	MAX.	MEMB	<ol><li>FORCE</li></ol>	MAX	
	(LBS)	(PI	_F) (	CSI (LC)	UNBRAC		(LBS)	CSI (LC)	
FR-TO		FROM	TO		LENGTH	FR-TC	)		
1- 2	0 / 29	-77.3	-77.3	0.03(1)	10.00	9- 3	-641 / 1682	0.14 (6)	
2- 3	-3502 / 1127	-77.3	-77.3	0.13 (7)	5.85	3-8	-1776 / 815	0.68 (3)	
	-2325 / 819	-77.3	-77.3	0.12 (7)	6.25	8- 5	-1496 / 4464	0.29(3)	
	-2325 / 819	-77.3		0.12 (7)		5- 7	-4145 / 1654	0.94(3)	
	-202 / 287			0.09(7)		2- 9	-1048 / 3465	0.23 (4)	
7- 6	-176 / 181	0.0	0.0	0.30 (5)	6.25				
10- 2	-4049 / 1278	0.0	0.0	0.23(4)	7.03				
10-9	-703 / 501	-620.2	-620.2	0.28(3)	6.25				
9- 8	-1237 / 2959	-525.3	-525.3	0.36(3)	6.25				
8- 7	-784 / 1904	-525.3	-525.3	0.30(3)	6.25				

RENCE VELOCITY PRESSURE OF { 9.0} PSF AT TE GRADE AND USING EXTERNAL PEAK
I WIND FORCE RESISTING SYSTEM INTERNAL
TEGORY 2. BUILDING MAY BE TO CATE ON
TO BE LOCATED AT LEAST (COLOR OF THE CATE OF THE OF THE CATE OF T

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

TOWN OF MILTON MAR 29, 2017 17-4978 **BUILDING DIVISION** 

LL = DL = LL = DL = AD = CH. PSF 3.0 PSF 7.0 **PSF** TOTAL LOAD 33.3 PSF

### SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder START DISTANCE = 0-0 START SPAN CARRIED = 27-10-8 END DISTANCE = 4-7-8 END SPAN CARRIED = 27-10-8 END WALL WIDTH = 5-8 APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

GIRDER TYPE: CStdGirder START DISTANCE = 4-7-8 START SPAN CARRIED = 23-10-8 END DISTANCE = 14-3-4 FND SPAN CARRIED = 23-10-8 END WALL WIDTH = 5-8
APPLIED TO FRONT SIDE OF BOTTOM CHORD. - ADDT'L LOADS BASED ON 55 % OF GSL

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

PART 9 OF OBC 2012, BCBC 2012, ABC 2014

- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.47")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.47") CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.30 (6-7:5) , BC=0.36 (8-9:3) , WB=0.94 (5-7:3) , SSI=0.35 (9-10:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT

RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN

MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (7) (INPUT = 0.90 JSI METAL= 0.33 (8) (INPUT = 1.00) JOB NAME TRUSS NAME

TW0317-048

**GIRD05** 

QUANTITY PLY

2

JOB PESC TR-GREENPARK-LECCO RIDGE-BLOCK 327 TRUSS DESC.

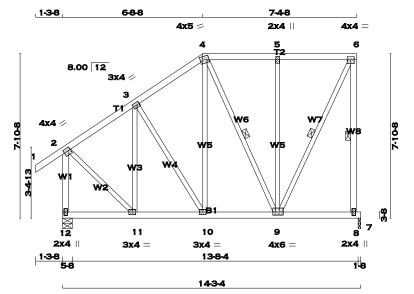
DRWG NO.

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SCALE = 1:55.2

Kott Lumber Uxbridge, Stouffville, ON, TW

Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:21 2017 Page 1 ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-WO9sIUMsugCPsIGQcHO2XTaDCF9PxQ9D817UzTzcJK



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR 4 6 2x4 DRY No.2 No.2 SPF SPF 2x4 DRY 2x4 DRY No 2 SPF 6 2 7 2x4 No.2 12 -2x4 DRY No.2 SPF ALL WEBS DRY No.2 SPF 2x3 EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)
JT TYPE PLATES
2 TMVW-t MT20 W LEN Y Y X 1.75 1.00 4.0 4.0 3.0 4.0 2.0 4.0 5.0 1.50 1.50 1.75 1.50 TMWW-t MT20 TTWW-m 4.0 TMW+w MT20 TMVW-t MT20 4.0 1.75 2.00 4.0 BMV+p MT20 4.0 RMWWW-t MT20 6.0 175 150 BMWW-t 3.0 4.0 MT20 11 3.0 4.0 BMWW-t MT20 1.50 1.75 BMV1+p MT20 2.0 4.0

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEAH	RINGS						
	FACTOR	ED	MAXIMUN	/ FACTO	ORED	INPUT	REQRD
	<b>GROSS RE</b>	ACTION	GROSS F	REACTIO	BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
12	1044	0	1134	442	-457	5-8	5-8
7	923	0	1029	0	-498	1-8	1-8

PROVIDE ANCHORAGE AT BEARING JOINT 12 FOR 457 LBS FACTORED UPLIF PROVIDE ANCHORAGE AT BEARING JOINT 7 FOR 498 LBS FACTORED UPLIFT

PROVIDE FOR 442 LBS FACTORED HORIZONTAL REACTION AT JOINT 12

UNFACTORED REACTIONS

	1ST LCASE	MAX./I	MIN. COMPO	NENT REACTION	ONS		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
12	730	524 / 0	0/0	0/0	225 / -459	206 / 0	0/0
7	648	452 / 0	0/0	0/0	264 / -482	197 / 0	0/0
HOR	IZONTAL RE	ACTIONS					
12		0/0	0/0	0/0	316 / -239	0/0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 12, 7

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 6-8. DBS = 16-0-0 . CBF = 89 LBS. 1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 4-9, 6-9. DBS = 20-0-0 . CBF = 54 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING TOTAL LOAD CASES: (11)

	C H O R D S MAX. FACTORED FACTORED					W E B S MAX. FACTORED			
MEMB.	FORCE	VERT. LO	AD LC	1 MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PL	.F) (	CSI (LC)	UNBRAC		(LBS)	CSI (LC)	
FR-TO		FROM	TO		LENGTH	I FR-TO			
1- 2	0/29	-77.3	-77.3	0.11(1)	10.00	11-3	-234 / 108	0.12 (4)	
2- 3	-643 / 354	-77.3	-77.3	0.22(7)	6.25	3-10	-141 / 233	0.10 (3)	
3- 4	-623 / 432	-77.3	-77.3	0.23 (7)	6.25	10- 4	-190 / 284	0.22 (7)	
4- 5	-441 / 365	-77.3	-77.3	0.18 (7)	6.25	4- 9	-185 / 159	0.09(3)	
5- 6	-441 / 366	-77.3	-77.3	0.18 (7)	6.25	9- 5	-365 / 283	0.42 (3)	
8- 6	-988 / 527	0.0	0.0	0.45 (7)	6.25	9-6	-486 / 969	0.23 (7)	
12- 2	-1050 / 455	0.0	0.0	0.20 (1)	7.65	2-11	-167 / 711	0.16 (1)	
12-11	-382 / 306	-54.1	-54.1	0.10(3)	6.25				
11-10	-395 / 580	-54.1	-54.1	0.18(1)	6.25				
10-9	-300 / 510	-54.1	-54.1	0.25 (1)	6.25				
9-8	-58 / 149	-54.1	-54.1	0.27(1)	6.25				
8- 7	0/0	-54.1	-54.1	0.26 (1)	10.00				

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK
COELEMENTS, BOOK BY THE WIND FORCE RESISTING SYSTEM).INTERNAL WIN {OPI TEGORY 2). BUILDING MAY BE LOCATED ON TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

RECEIVED TOWN OF MILTON

MAR 29, 2017 17-4978 **BUILDING DIVISION** 

TOTAL WEIGHT = 2 X 85 = 170 lb **DESIGN CRITERIA** 

SPECIFIED LOADS: LL = DL = LL = DL = AD = CH. PSF 3.0 PSF 7.0 PSF

SPACING = 24.0 IN. C/C

TOTAL LOAD

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

33.3 PSF

GIRDER TYPE: CStdGirder START DISTANCE = 0-0 START SPAN CARRIED = 4-0-0 END DISTANCE = 14-3-4 END SPAN CARRIED = 4-0-0 END WALL WIDTH = 5-8
APPLIED TO FRONT SIDE OF BOTTOM CHORD. - ADDT'L LOADS BASED ON 55 % OF GSL

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012, BCBC 2012, ABC 2014 CSA 086-09 - TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.48")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.48") CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.45 (6-8:7) , BC=0.27 (8-9:1) , WB=0.42 (5-9:3) , SSI=0.79 (7-8:1)

COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

618 354 1667 822 2284 1656

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

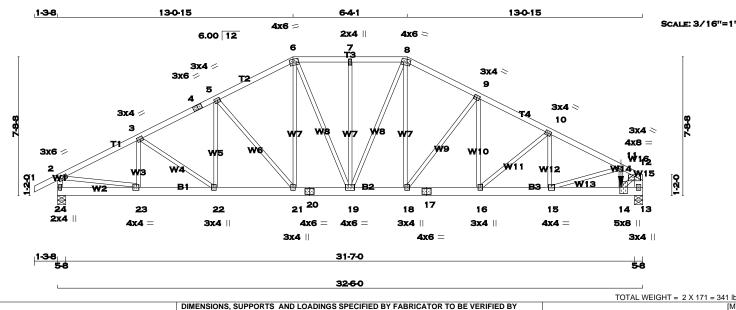
MT20

JSI GRIP= 0.89 (11) (INPUT = 0.90) JSI METAL= 0.29 (2) (INPUT = 1.00)



JOB PESC PREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY PLY DRWG NO. Page 14 of 159 TRUSS DESC 2 TW0317-048 TW0317-048 **GIRD06** Kott Lumber Uxbridge, Stouffville, ON, TW

Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:22 2017 Page 1 ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-\_ajEWqNUf\_KFTSrcA\_vH3g7SmfS9gqhMNht2WwzcJK7



<u>LUMBER</u>				
N. L. G. A. R	ULES			
CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 6	2x4	DRY	No.2	SPF
6 - 8	2x4	DRY	No.2	SPF
8 - 12	2x4	DRY	No.2	SPF
24 - 2	2x4	DRY	No.2	SPF
13 - 12	2x6	DRY	No.2	SPF
24 - 20	2x6	DRY	No.2	SPF
20 - 17	2x6	DRY	No.2	SPF
17 - 13	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
24 - 2 13 - 12 24 - 20 20 - 17 17 - 13	2x4 2x6 2x6 2x6 2x6 2x6	DRY DRY DRY DRY DRY	No.2 No.2 No.2 No.2 No.2 No.2	SPF SPF SPF SPF SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF <u>2</u> TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORD	S #ROWS	SURFACE	LOAD(PLF)							
		SPACING (IN)								
TOP CH	TOP CHORDS: (0.122"X3") SPIRAL NAILS									
1-4	1 `	12	TOP							
4-6	1	12	TOP							
6-8	1	12	TOP							
8- 12	1	12	TOP							
24- 2	1	12	TOP							
13- 12	2	12	TOP							
BOTTON	A CHORDS	: (0.122"X3") SPIRAL	NAILS							
24- 20	2	12	TOP							
20- 17	2	12	TOP							
17- 13	2	12	SIDE(0.0)							
WEBS:	, ,									
2x3	` 1 ′	6								

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.



DIMENSIONS, SUPP	ORTS AND LOADINGS	SPECIFIED BY FABRICA	TOR TO BE VERIFIED BY
<b>BUILDING DESIGNE</b>	ER .		
DEADINGS			

DEA	BEARINGS											
	FACTOR	RED	MAXIMUM FACTORED			INPUT	REQRD					
	GROSS RE	ACTION	GROSS I	REACTIO	BRG	BRG						
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX					
24	1783	0	1850	207	-857	5-8	5-8					
13	5073	0	5716	0	-2586	5-8	5-8					

PROVIDE ANCHORAGE AT BEARING JOINT 24 FOR 857 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 13 FOR 2586 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES

PROVIDE FOR 207 LBS FACTORED HORIZONTAL REACTION AT JOINT 24

# UNFACTORED REACTIONS

	1ST LCASE	MAX./	MIN. COMPO	NENT REACTI	ONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL			
24	1249	887 / 0	0/0	0/0	168 / -845	362 / 0	0/0			
13	3560	2490 / 0	0/0	0/0	1609 / -2535	1070 / 0	0/0			
HORIZONTAL REACTIONS										
24		0/0	0/0	0/0	148 / -121	0/0	0 /0			

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 24, 13

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.29 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (11)

СН	ORDS					W E	BS		
MAX	. FACTORED	FACTO	RED				MAX. FACTO	DRED	
ИЕМВ.	FORCE	VERT. LO	AD LC	1 MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PL	.F) (	CSI (LC)	<b>UNBRAC</b>	)	(LBS)	CSI (LC)	
R-TO		FROM	TO		LENGTH	FR-TO			
1-2	0 / 23	-77.3	-77.3	0.06(1)	10.00	23-3	-347 / 243	0.03(1)	
2-3	-2553 / 1182	-77.3	-77.3	0.19 (7)	5.60	3-22	-45 / 143	0.02(7)	
3- 4	-2535 / 1234	-77.3	-77.3	0.19 (7)	5.62	22-5	-34 / 115	0.02 (11)	
4- 5	-2535 / 1234	-77.3	-77.3	0.19 (7)	5.62	5-21	-466 / 376	0.19 (3)	
5-6	-2224 / 1139	-77.3	-77.3	0.18 (7)	5.90	21-6	-239 / 421	0.12 (7)	
6- 7	-2124 / 1118	-77.3	-77.3	0.13 (7)	6.05	6-19	-284 / 360	0.18 (8)	
7-8	-2124 / 1118	-77.3	-77.3	0.13 (7)	6.05	19- 7	-300 / 235	0.15 (3)	
8- 9	-2360 / 1211	-77.3	-77.3	0.17 (8)	5.80	19-8	-128 / 91	0.08 (7)	
9-10	-2953 / 1437	-77.3	-77.3	0.19(8)	5.32	18-8	-423 / 784	0.22 (8)	
10-11	-3766 / 1756	-77.3	-77.3	0.20 (8)	4.82	18- 9	-892 / 582	0.36(3)	
11-12	-5114 / 2332	-77.3	-77.3	0.20 (8)	4.29	16- 9	-287 / 650	0.07(1)	
24- 2	-1795 / 870	0.0	0.0	0.10(1)	7.81	16-10	-967 / 558	0.21 (3)	
13-12	-5278 / 2380	0.0	0.0	0.17(1)	6.43	15-10	-244 / 662	0.07(3)	
						15-11	-1251 / 643	0.18 (3)	
24-23	-192 / 169	-17.5	-17.5	0.03(1)	6.25	14-11	-399 / 865	0.09(3)	
23-22	-1135 / 2328	-17.5	-17.5	0.17(1)	6.25	2-23	-957 / 2320	0.28 (1)	
22-21	-1018 / 2292	-17.5	-17.5	0.15(1)	6.25	14-12	-2448 / 5423	0.60(1)	
21-20	-777 / 1996	-17.5	-17.5	0.13(1)	6.25				
20-19	-777 / 1996	-17.5	-17.5	0.13(1)	6.25				
19-18	-762 / 2118	-17.5	-17.5	0.14(1)	6.25				
18-17	-1013 / 2647	-17.5	-17.5	0.18(1)	6.25				
17-16	-1013 / 2647	-17.5	-17.5	0.18(1)	6.25				
16-15	-1439 / 3385	-17.5	-17.5	0.23(1)	6.25				
15-14	-2046 / 4566	-17.5	-17.5	0.43(1)	6.25				
14-13	-8 / 16	-17.5	-17.5	0.17 (3)	10.00				
EACTO	RED CONCENT	DATEDIO	VDS (I	DC)					
FACIO	KED CONCEIN	INA IED LO	) SUA	_00)					

JT LOC LC1 MAX-MAX+

FACE DIR VERT -4324 1934 FRONT

TOWN OF MILTON WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURFAR (299) 25F AT WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURAL (201) 251 AT (40-0) FI-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESIS ING SYSTEM): 110 PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST LICENTAL WIND FROM EAVE.

**RECEIVED** 

### **DESIGN CRITERIA**

\*\*\* SPECIAL LOADS ANALYSIS \*\*\*
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS: TOP CH.

LL = 23.3 DL = 3.0 LL = 0.0 PSF PSF BOT CH. DL PSF TOTAL LOAD PSF 33.3

#### SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

\*\*\* NON STANDARD GIRDER \*\*\* ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014

- CSA 086-09 - TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.08°) CALCULATED VERT. DEFL.(LL)= L/ 999 (0.06°) ALLOWABLE DEFL.(TL)= L/360 (1.08°) CALCULATED VERT. DEFL.(TL)= L/999 (0.10°)

CSI: TC=0.20 (10-11:8) , BC=0.43 (14-15:1) , WB=0.60 (12-14:1) , SSI=0.13 (13-14:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

TRUSS MANUFACTURING PLANT

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (14) (INPUT = 0.90) JSI METAL= 0.59 (14) (INPUT = 1.00)

TR-GREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY PLY DRWG NO. TRUSS DESC. 2 TW0317-048 GIRD06

Kott Lumber Uxbridge, Stouffville, ON, TW

Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:22 2017 Page 2 ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-\_ajEWqNUf\_KFTSrcA\_vH3g7SmfS9gqhMNht2WwzcJK7

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TW0317-048

 
 PLATES
 (table is in inches)

 JT
 TYPE
 PLATES
 W
 LEN
 Y

 2
 TMVW-t
 MT20
 3.0
 6.0
 3, 5, 9, 10, 11 3 TMWW-t 4 TS-t 6 TTWW-m 7 TMW+w 4.0 1.50 1.75 6.0 6.0 1.75 2.25 4.0 MT20 3.0 3.0 4.0 2.0 4.0 4.0 3.0 5.0 4.0 TS-t TTWW-m MT20 MT20 TMW+w TTWW-m TMVW-p MT20 4.0 6.0 1.75 2.25 8.0 1.00 4.00 4.0 8.0 4.25 2.00 4.0 MT20 MT20 MT20 MT20 8 12 13 14 15 BMV1+p BMWW+t BMWW-t MT20 MT20 15 BMWW-t 16, 18, 21, 22 16 BMWW+t 17 BS-t 19 BMWWW-t 20 BS-t 23 BMWW-t 24 BMV1+p MT20 3.0 4.0 4.0 4.0 4.0 4.0 2.0 MT20 6.0 6.0 MT20 MT20 MT20 MT20 6.0 4.0 4.0 2.00 1.50

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

HANGERS NOTES

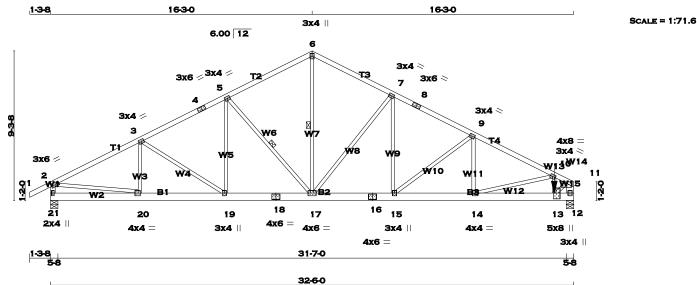
1) SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 4323.5 lbs FACTORED DOWN AND
1934.4 lbs FACTORED UP AT 31-3-8 ON BOTTOM
CHORD. DESIGN FOR UNSPECIFIED
CONNECTION(S) IS DELEGATED TO THE
BUILDING DESIGNER.



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

JOB PESC TR-GREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY DRWG NO Page 16 of 159 TRUSS DESC TW0317-048 TW0317-048 **GIRD07** Kott Lumber Uxbridge, Stouffville, ON, TW

Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:22 2017 Page 1 ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-\_ajEWqNUf\_KFTSrcA\_vH3g7SGfSagpdMNht2WwzcJK7



LUMBER				
N. L. G. A. R	ULES			
CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 6	2x4	DRY	No.2	SPF
6 - 8	2x4	DRY	No.2	SPF
8 - 11	2x4	DRY	No.2	SPF
21 - 2	2x4	DRY	No.2	SPF
12 - 11	2x6	DRY	No.2	SPF
21 - 18	2x6	DRY	No.2	SPF
18 - 16	2x6	DRY	No.2	SPF
16 - 12	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF <u>2</u> TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORD	S #ROWS		LOAD(PLF)
		SPACING (IN)	1
TOP CH	ORDS: (0.	122"X3") SPIŘAĹ	. NAILS
1-4	1	12	TOP
4-6	1	12	TOP
6-8	1	12	TOP
8- 11	1	12	TOP
21-2	1	12	TOP
12- 11	2	12	TOP
BOTTON	A CHORDS	: (0.122"X3") SF	PIRAL NAILS
21- 18	2	12	TOP
18- 16	2	12	TOP
16- 12	2	12	SIDE(0.0)
WEBS:	(0.122"X3")	SPIRAL NAILS	
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLE MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.



DIMENSIONS, SUPPORTS	AND LOADINGS SPEC	IFIED BY FABRICATO	R TO BE VERIFIED BY
BUILDING DESIGNER			
CADINOC			

BEA	RINGS						
	FACTO	RED	MAXIMU	M FACT	ORED	INPUT	REQRD
	GROSS R	EACTION	GROSS	REACTIO	N	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
21	1773	0	1829	248	-805	5-8	5-8
12	4831	0	5424	0	-2411	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 21 FOR 805 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 12 FOR 2411 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES

PROVIDE FOR 248 LBS FACTORED HORIZONTAL REACTION AT JOINT 21

## UNFACTORED REACTIONS

	151 LCASE	MAX./	MIN. COMPONI	<u>ENT REACTI</u>	ONS		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
21	1242	882 / 0	0/0	0/0	138 / -806	360 / 0	0/0
12	3391	2372 / 0	0/0	0/0	1483 / -2377	1019/0	0/0
HOR	IZONTAL RE	EACTIONS					
21		0/0	0/0	0/0	177 / -143	0/0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 21, 12

ח

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.37 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 5-17, 6-17. DBS = 20-0-0 . CBF = 83

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) TO EACH PLY USING (0.122°X3°) SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING TOTAL LOAD CASES: (11)

	HORDS				WE		
MA	X. FACTORED	FACTOR	RED			MAX. FACTO	RED
MEMB	. FORCE	VERT. LO	AD LC1 MAX	MAX.	MEMB.	FORCE	MAX
	(LBS)		F) CSI (LC)			(LBS)	CSI (LC)
FR-TO	)	FROM		LENGTH			
1- 2	0 / 23	-77.3	-77.3 0.06 (1)			-243 / 211	0.03 (1)
2- 3	-2584 / 1117	-77.3	-77.3 0.24 (7)			-256 / 275	0.09 (3)
3- 4	-2377 / 1088	-77.3	-77.3 0.23 (7)	5.68	19- 5	-87 / 233	0.03 (7)
4- 5	-2377 / 1088		-77.3 0.23 (7)			-659 / 509	0.15 (3)
5- 6	-1927 / 969	-77.3	-77.3 0.22 (7)	6.15	17- 6	-652 / 1381	0.17 (8)
6- 7	-1933 / 973		-77.3 0.20 (8)	6.18		-982 / 667	0.67 (4)
7-8	-2586 / 1192	-77.3	-77.3 0.22 (8)			-285 / 640	0.10 (8)
8- 9	-2586 / 1192	-77.3	-77.3 0.22 (8)		15- 9	-980 / 603	0.34 (3)
9-10	-3445 / 1530	-77.3	-77.3 0.23 (8)		14- 9	-199 / 585	0.06 (3)
10-11	-4901 / 2193	-77.3	-77.3 0.23 (8)		14-10 -	1347 / 750	0.30 (3)
21-2	-1773 / 825	0.0	0.0 0.09 (1)	7.81	13-10	-357 / 774	0.08 (3)
12-11	-5034 / 2212	0.0	0.0 0.16 (1)	6.54	2-20	-879 / 2340	0.28 (1)
					13-11 -	2323 / 5220	0.58 (1)
21-20	-233 / 210	-17.5	-17.5 0.03 (8)	6.25			
20-19	-1104 / 2368	-17.5	-17.5 0.17 (1)	6.25			
19-18	-875 / 2155	-17.5	-17.5 0.16 (1)	6.25			
18-17	-875 / 2155	-17.5	-17.5 0.16 (1)	6.25			
17-16	-790 / 2321	-17.5	-17.5 0.17 (1)	6.25		D	EOEIVED.
16-15	-790 / 2321	-17.5	-17.5 0.17 (1)	6.25		K	ECEIVED
15-14	-1218 / 3096	-17.5	-17.5 0.22 (1)	6.25		TOW	N OF MILTON
14-13	-1941 / 4395	-17.5	-17.5 0.40 (1)	6.25			
13-12	-8 / 16	-17.5	-17.5 0.15 (3)	10.00		MA	AR 29, 2017
FACT	ORED CONCEN	TRATED LO	ADS (LBS)				17-4978
JT	LOC. LC	1 MAX-	MAX+ F	ACE I	DIR.	YPE	
13	31-3-8 -341	7 -4028	1802 FR	ONT VI	ERT	τÞτ⊠UIL□	DING DIVISION

### **DESIGN CRITERIA**

\*\*\* SPECIAL LOADS ANALYSIS \*\*\*
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER

TOTAL WEIGHT = 2 X 158 = 316 lb

LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS: LL = 23.3 DL = 3.0 LL = 0.0 TOP CH. PSF PSF PSF BOT CH. DL PSF TOTAL LOAD PSF 33.3

### SPACING = 24.0 IN. C/C

\*\*\* NON STANDARD GIRDER \*\*\*
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014 - CSA 086-09 - TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.08")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (1.08") CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.24 (2-3:7), BC=0.40 (13-14:1), WB=0.67 (7-17:4), SSI=0.11 (12-13:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (2) (INPUT = 0.90 ) JSI METAL= 0.57 (13) (INPUT = 1.00 )

POBPES GREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY PLY DRWG NO. TRUSS DESC. 2 TW0317-048 **GIRD07** 

Kott Lumber Uxbridge, Stouffville, ON, TW

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TW0317-048

 
 PLATES
 (table is in inches)

 JT
 TYPE
 PLATES
 W
 LEN
 Y

 2
 TMVW-t
 MT20
 3.0
 6.0
 , 5, 7, 9, 10 TMWW-t 3, 3 4 6 8 11 12 13 14 15 4.0 6.0 4.0 MT20 1.50 1.75 3.0 3.0 3.0 4.0 3.0 5.0 4.0 4.0 4.0 4.0 2.0 TS-t MT20 TTW+p MT20 TS-t TMVW-p BMV1+p BMWW+t MT20 6.0 8.0 4.0 8.0 4.0 4.0 MT20 1.00 4.00 MT20 MT20 4.25 2.00 BMWW-t BMWW+t MT20 MT20 BS-t BMWWW-t MT20 MT20 6.0 18 19 20 21 BS-t BMWW+t BMWW-t MT20 6.0 4.0 MT20 4.0 4.0 2.00 1.50 MT20 MT20 BMV1+p

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 4027.9 lbs FACTORED DOWN AND
1802.2 lbs FACTORED UP AT 31-3-8 ON BOTTOM
CHORD. DESIGN FOR UNSPECIFIED
CONNECTION(S) IS DELEGATED TO THE
DI III DINIC DESIGNER BUILDING DESIGNER.

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM].INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

JOB NAME TRUSS NAME

Kott Lumber Uxbridge, Stouffville, ON, TW

TW0317-048

**GIRD08** 

PLY

QUANTITY

JOB PESC TR-GREENPARK-LECCO RIDGE-BLOCK 327 TRUSS DESC

DRWG NO

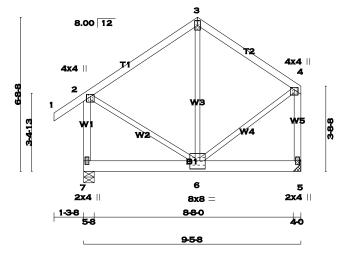
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SCALE = 1:50.2

TOTAL WEIGHT = 2 X 52 = 104 lb



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR 3 2x4 DRY No.2 No.2 SPF SPF DRY 2x4 2 2x4 DRY No 2 SPF No.2 5 2x6 DRY No.2 SPF ALL WEBS 2x3 DRY SPF No.2 EXCEPT

DRY: SEASONED LUMBER

DESIGN CONSISTS OF  $\underline{\mathbf{2}}$  TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS SURFACE LOAD(PLF) SPACING (IN) TOP CHORDS : (0.122"X3") SPIRAL NAILS TOP TOP 3-4 12 7- 2 5- 4 12 TOP TOP BOTTOM CHORDS: (0.122"X3") SPIRAL NAILS SIDE(399.4) WEBS: (0.122"X3") SPIRAL NAILS

NAILS TO BE DRIVEN FROM ONE SIDE ONLY

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING.
REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Υ	Χ
2	TMVW+p	MT20	4.0	4.0	1.25	2.00
3	TTW+p	MT20	3.0	5.0		
4	TMVW+p	MT20	4.0	4.0	1.25	2.00
5	BMV1+p	MT20	2.0	4.0		
6	BMWWW-t	MT20	8.0	8.0	4.25	4.00
7	RM\/1±n	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY **BUILDING DESIGNER** 

BEA	RINGS						
	FACTOR	RED	MAXIMUN	M FACTO	ORED	INPUT	REQRD
	<b>GROSS RE</b>	ACTION	GROSS F	REACTIO	N	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	<b>UPLIFT</b>	IN-SX	IN-SX
7	3774	0	4359	333	-1380	5-8	5-8
5	3668	0	4264	0	-1370	HANGER E	BY OTHERS
						MIN. SEAT	SIZE: 4-0

PROVIDE ANCHORAGE AT BEARING JOINT 7 FOR 1380 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 5 FOR 1370 LBS FACTORED UPLIFT

ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES. SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 333 LBS FACTORED HORIZONTAL REACTION AT JOINT 7

UNFACTORED REACTIONS

	1ST LCASE	MAX./	MIN. COMPON	NENT REACT	IONS		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
7	2646	1865 / 0	0/0	0/0	1463 / -1488	782 / 0	0/0
5	2574	1801 / 0	0/0	0/0	1491 / -1476	773 / 0	0/0
HOR 7	IZONTAL RE	EACTIONS 0/0	0/0	0/0	238 / -224	0/0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 7

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.68 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (11)

	O R D S	FACTO	RED			WE	B S MAX. FACTO	RED	
MEMB.	FORCE	VERT. LC	OAD LC	1 MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PI	_F)	CSI (LC)	UNBRAC	0	(LBS)	CSI (LC)	
FR-TO		FROM	TO		LENGTH	FR-TO			
1-2	0 / 29	-77.3	-77.3	0.06(1)	10.00	6-3	-681 / 2196	0.24 (4)	
2-3	-2339 / 834	-77.3	-77.3	0.25 (7)	5.68	2-6	-738 / 2283	0.25 (4)	
3- 4	-2340 / 849	-77.3	-77.3	0.21 (8)	5.73	6- 4	-777 / 2416	0.26(3)	
7-2	-2764 / 922	0.0	0.0	0.24 (4)	6.92				
5- 4	-2899 / 978	0.0	0.0	0.30 (3)	6.81				
7- 6	-274 / 286	-698.2	-698.2	0.68 (3)	6.25				
6- 5	-30 / 65			0.68 (4)					

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT WIND LOAD APPLIED IS DENIVED FROM REPERSINCE VELOCITY FRESSURE OF (\$3.0) FOR AL (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

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### **DESIGN CRITERIA**

SPECIFIED LOADS: LL =
DL =
LL =
DL =
AD = CH. PSF 3.0 PSF 7.0 PSF TOTAL LOAD 33.3 PSF

### SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder START DISTANCE = 0-0 START SPAN CARRIED = 31-2-0 END DISTANCE = 9-5-8 END SPAN CARRIED = 31-2-0 END WALL WIDTH = 5-8 APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09

- TPIC 2011

(55 % OF 27 2 P.S.F. G.S.I. PLUS 8 4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.32")
CALCULATED VERT. DEFL.(LL)= L/999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.32")
CALCULATED VERT. DEFL.(TL)= L/999 (0.10")

CSI: TC=0.30 (4-5:3) , BC=0.68 (6-7:3) , WB=0.26 (4-6:3) , SSI=0.71 (6-7:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (4) (INPUT = 0.90) JSI METAL= 0.30 (6) (INPUT = 1.00) TW0317-048

Kott Lumber Uxbridge, Stouffville, ON, TW

**GIRD09** 

TRUSS DESC.

QUANTITY

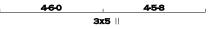
PLY

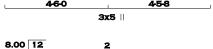
POBPES GREENPARK-LECCO RIDGE-BLOCK 327

DRWG NO.

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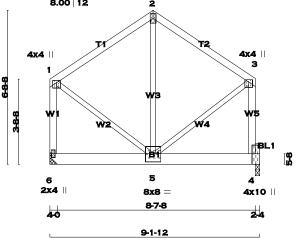
Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:23 2017 Page 1 ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-SnHcjAO6QHS65cQpkhQWcufcO3lxPNTWbLcb2MzcJK6







TOTAL WEIGHT = 2 X 50 = 99 lb



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR 2 2x4 DRY No.2 No.2 SPF SPF 2x4 DRY 6 2x4 DRY No 2 SPF No.2 6 2x6 DRY No.2 SPF BEARING BLOCKS BL1 DRY No.2 SPF ALL WEBS 2x3 DRY SPF No.2 **EXCEPT** 

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF  $\underline{2}$  TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORE	OS #ROWS	SURFACE	LOAD(PLF)
		SPACING (IN)	
TOP CH	HORDS: (0.1	22"X3") SPIRAL N	AILS
1-2	1	12	TOP
2-3	1	12	TOP
6- 1	1	12	TOP
4-3	1	12	TOP
вотто	M CHORDS	: (0.122"X3") SPIR	AL NAILS
6- 4	2	` 8	SIDE(399.4
WEBS:	(0.122"X3")	SPIRAL NAILS	
2x3	. 1 ´	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP

PLATES	(table is in inches)
IT TYPE	

FL	FLATES (table is ill lilicities)							
JT	TYPE	PLATES	W	LEN	Y X			
1	TMVW+p	MT20	4.0	4.0	1.25 2.00			
2	TTW+p	MT20	3.0	5.0				
3	TMVW+p	MT20	4.0	4.0	1.25 2.00			
4	BMVK1+t	MT20	4.0	10.0	Edge 1.75			
5	BMWWW-t	MT20	8.0	8.0	4.25 4.00			
6	BMV1+p	MT20	2.0	4.0				



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEA	BEARINGS										
	FACTORED		MAXIMUM FACTORED			INPUT	REQRD				
	GROSS R	EACTION	GROSS	REACTIO	N	BRG	BRG				
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX				
6	3417	0	3974	308	-1261	HANGER I	BY OTHERS				
						MIN. SEAT	SIZE: 4-0				
4	3429	0	3987	0	-1263	2-4	2-4				

PROVIDE ANCHORAGE AT BEARING JOINT 6 FOR 1261 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 4 FOR 1263 LBS FACTORED UPLIFT

ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES. SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 308 LBS FACTORED HORIZONTAL REACTION AT JOINT 6

UNFACTORED REACTIONS

	1ST LCASE	MAX./	<u>MIN. COMPON</u>	ENT REACT	IONS		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
6	2398	1678 / 0	0/0	0/0	1393 / -1364	721 / 0	0/0
4	2406	1685 / 0	0/0	0/0	1395 / -1366	721 / 0	0/0
HOR 6	IZONTAL RE	EACTIONS 0/0	0/0	0/0	220 / -220	0/0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 4 BEARING SIZE FACTOR = 1.08 AT JNT(S) 4 ( BASED ON SUPPORT DEPTH = 1-8 )

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.04 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (11)

СН	ORDS	WEBS							
MAX	X. FACTORED	FACTO	RED		MAX. FACTORED				
MEMB.	FORCE	VERT. LC	DAD LC	1 MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PI	LF) (	CSI (LC)	<b>UNBRAC</b>		(LBS)	CSI (LC)	
FR-TO		FROM	TO		LENGTH	FR-TO			
1- 2	-2032 / 739	-77.3	-77.3	0.21 (7)	6.04	5- 2	-586 / 1875	0.20(4)	
2-3	-2031 / 739	-77.3	-77.3	0.20 (8)	6.04	1- 5	-684 / 2098	0.23(4)	
6- 1	-2541 / 849	0.0	0.0	0.26 (4)	7.16	5-3	-729 / 2255	0.24(3)	
4- 3	-2649 / 882	0.0	0.0	0.28 (3)	7.05				
6- 5	-244 / 277	-698.2	-698.2	0.58(3)	6.25				
5- 4	-142 / 104	-698.2	-698.2	0.58 (4)	6.25				

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT WIND LOAD APPLIED IS DERIVED FROM REPERSING VELOCITY FRESTURE OF (\$3.0) FOR AL (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

RECEIVED TOWN OF MILTON MAR 29, 2017 17-4978 **BUILDING DIVISION** 

#### **DESIGN CRITERIA**

SPECIFIED LOADS:								
TOP	CH.	LL	=	23.3	PSF			
		DL	=	3.0	PSF			
BOT	CH.	LL	=	0.0	PSF			
		DL	=	7.0	PSF			
TOTA	L LO	AD	=	33.3	PSF			

### SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder START DISTANCE = 0-0 START SPAN CARRIED = 31-2-0 END DISTANCE = 9-1-2 END SPAN CARRIED = 31-2-0 END WALL WIDTH = 5-8 APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- TPIC 2011

- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09

(55 % OF 27 2 P.S.F. G.S.I. PLUS 8 4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.29\*) CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04\*) ALLOWABLE DEFL.(TL)= L/360 (0.29\*) CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07\*)

CSI: TC=0.28 (3-4:3) , BC=0.58 (5-6:3) , WB=0.24 (3-5:3) , SSI=0.65 (5-6:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN MAX MIN

MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (3) (INPUT = 0.90) JSI METAL= 0.28 (5) (INPUT = 1.00) JOB NAME

Kott Lumber Uxbridge, Stouffville, ON, TW

TW0317-048

TRUSS NAME **GIRD10** 

QUANTITY PLY 2

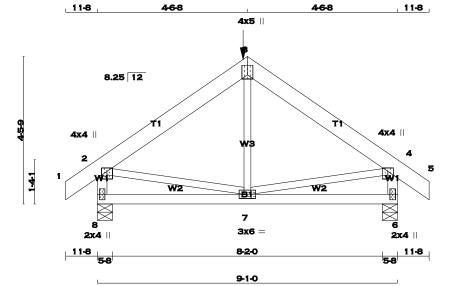
JOB PR-GREENPARK-LECCO RIDGE-BLOCK 327 TRUSS DESC.

DRWG NO.

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SCALE = 1:34.9

Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:23 2017 Page 1 ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-SnHcjAO6QHS65cQpkhQWcufeo3spPPdWbLcb2MzcJK6



TOTAL WEIGHT = 2 X 47 = 94 lb

<u>LUMBER</u>										
N. L. G. A. R	N. L. G. A. RULES									
CHORDS	SIZE		LUMBER	DESCR.						
1 - 3	2x6	DRY	No.2	SPF						
3 - 5	2x6	DRY	No.2	SPF						
8 - 2	2x4	DRY	No.2	SPF						
6 - 4	2x4	DRY	No.2	SPF						
8 - 6	2x4	DRY	No.2	SPF						
ALL WEBS	2x3	DRY	No.2	SPF						
EXCEPT										

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Υ	X
2	TMVW+p	MT20	4.0	4.0	1.25	2.00
3	TTW+p	MT20	4.0	5.0	3.50	2.00
4	TMVW+p	MT20	4.0	4.0	1.25	2.00
6	BMV1+p	MT20	2.0	4.0		
7	BMWWW-t	MT20	3.0	6.0		
8	BMV1+p	MT20	2.0	4.0		
	•					

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 316.3 lbs FACTORED DOWN AND 223.2 lbs FACTORED UP AT 4-6-8 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
BEARINGS

	BLARINGO									
	FACTORED		MAXIMU	M FACT	INPUT	REQRD				
	GROSS RI	EACTION	GROSS REACTION			BRG	BRG			
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX			
8	657	0	723	0	-320	5-8	5-8			
6	657	0	723	-205	-320	5-8	5-8			

PROVIDE ANCHORAGE AT BEARING JOINT 8 FOR 320 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 6 FOR 320 LBS FACTORED UPLIFT

PROVIDE FOR 205 LBS FACTORED HORIZONTAL REACTION AT JOINT 6

UNFACTORED REACTIONS

	1ST LCASE	MAX./I	MIN. COMPON	<u>NENT REACTIO</u>	DNS		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
8	459	335 / 0	0/0	0/0	164 / -309	124 / 0	0/0
6	459	335 / 0	0/0	0/0	164 / -309	124 / 0	0/0
HOR 6	IZONTAL REA	ACTIONS 0/0	0/0	0/0	146 / -146	0/0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 8, 6

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED

FROM EAVE.

<u>LOADING</u> TOTAL LOAD CASES: (11)

	DRDS					WE		
MAX.	FACTORED	FACTOR	Eυ				MAX. FACTO	RED
MEMB.	FORCE	VERT. LOA	AD LC1	MAX	MAX.	MEMB.	FORCE	MAX
	(LBS)	(PLF	-) (	CSI (LC)	<b>UNBRA</b>	0	(LBS)	CSI (LC)
FR-TO		FROM 1	ľО		LENGTH	FR-TO		
1- 2	0 / 24	-77.3	-77.3	0.04(1)	10.00	7-3	-57 / 131	0.04 (11)
2-3	-569 / 305	-77.3	-77.3	0.19(7)	6.25	2- 7	-175 / 485	0.10 (4)
3- 4	-569 / 305	-77.3	-77.3	0.19(8)	6.25	7- 4	-175 / 485	0.10 (3)
4- 5	0 / 24	-77.3	-77.3	0.04(1)	10.00			
8- 2	-683 / 347	0.0	0.0	0.07(1)	7.81			
6- 4	-683 / 347	0.0	0.0	0.07 (1)	7.81			
8- 7	-8 / 18	22.2	22.2	0.14 (11	\ 10.00			
7- 6	-187 / 196	-22.3	-22.3	0.14 (11	) 6.25			
FACTOR	FACTORED CONCENTRATED LOADS (LBS)							
JT	LOC. LC1	MAX-	MAX-	+ F/	ACE I	DIR.	TYPE	
3	4-6-8 -241	-316	223	3 FR	IV TNC	ERT	TOTAL	

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF ( 9.0) PSF AT WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (\$3.0) PSP AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY

T.L. WISE 100083566 100083566 WCE OF ONTAR March 10, 2017

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

**RECEIVED** TOWN OF MILTON MAR 29, 2017 17-4978 **BUILDING DIVISION** 

### **DESIGN CRITERIA**

TOTAL LOAD

4

SPECIFIED LOADS: LL = DL = LL = DL = AD = CH. PSF 3.0 PSF 7.0 PSF

### SPACING = 24.0 IN. C/C

GIRDER TYPE: CPrimeHip SIDE SETBACK = 4-6-8 END SETBACK = 3-6-11 END WALL WIDTH = 5-8
CORNER FRAMING TYPE: CONVENTIONAL END JACK TYPE: CONVENTIONAL APPLIED TO FRONT SIDE - ADDT'L LOADS BASED ON 55 % OF GSL.

33.3 PSF

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09

- TPIC 2011

(55 % OF 27 2 P.S.F. G.S.I. PLUS 8 4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.30°) CALCULATED VERT. DEFL.(LL)= L/999 (0.01°) ALLOWABLE DEFL.(TL)= L/360 (0.30°) CALCULATED VERT. DEFL.(TL)= L/999 (0.02°)

CSI: TC=0.19 (2-3:7) , BC=0.14 (7-8:11) , WB=0.10 (2-7:4) , SSI=0.10 (2-3:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN MAX MIN

MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (7) (INPUT = 0.90) JSI METAL= 0.17 (2) (INPUT = 1.00) JOB NAME TRUSS NAME

Kott Lumber Uxbridge, Stouffville, ON, TW

TW0317-048

**GIRD11** 

QUANTITY

PLY

3

TRUSS DESC

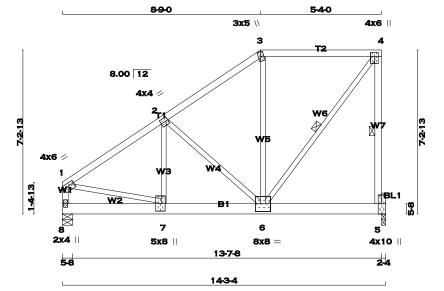
JOB PESC PREENPARK-LECCO RIDGE-BLOCK 327

DRWG NO.

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SCALE = 1:50.8

Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:24 2017 Page 1 ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-wzr\_xWPkBbazjm??IPxl85CmoT9B8kxfq?M8aozcJK5



TOTAL WEIGHT = 3 X 75 = 226 lb

LUMBER										
N. L. G. A. R	ULES									
CHORDS	SIZE	L	UMBER	DESCR.						
1 - 3	2x4	DRY	No.2	SPF						
3 - 4	2x4	DRY	No.2	SPF						
5 - 4	2x4	DRY	No.2	SPF						
8 - 1	2x4	DRY	No.2	SPF						
8 - 5	2x6	DRY	2100F 1.8E	SPF						
BEARING BI	OCKS									
BL1	2x6	DRY	No.2	SPF						
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF						

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF  $\underline{\mathbf{3}}$  TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORL	JS #ROWS	SURFACE	LOAD(PLF)					
		SPACING (IN)						
TOP CI	TOP CHORDS : (0.122"X3") SPIRAL NAILS							
1-3	1	12	TOP					
3- 4	1	12	TOP					
4- 5	1	12	TOP					
8- 1	1	12	TOP					
BOTTO	M CHORDS	: (0.122"X3") SPIRAL NAILS						
8- 5	2	4	SIDE(671.2)					
WEBS : (0.122"X3") SPIRAL NAILS								

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

<u>FL</u>	PLATES (table is in inches)									
JT	TYPE	PLATES	W	LEN	Υ	X				
1	TMVW-t	MT20	4.0	6.0	1.50	3.00				
2	TMWW-t	MT20	4.0	4.0	2.00	1.00				
3	TTW+m	MT20	3.0	5.0	2.50	1.25				
4	TMVW+p	MT20	4.0	6.0	2.25	1.75				
5	BMVK1+t	MT20	4.0	10.0	Edge	1.75				
6	BMWWW-t	MT20	8.0	8.0	4.25	4.00				



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY **BUILDING DESIGNER** 

BEARINGS									
FACTO	RED	MAXIMU	M FACT	ORED	INPUT	REQRD			
GROSS R	EACTION	GROSS	REACTIO	N	BRG	BRG			
VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX			
6652	0	7752	0	-2564	2-4	2-4			
6641	0	7728	383	-2465	5-8	5-8			
	FACTO GROSS R VERT 6652	FACTORED GROSS REACTION VERT HORZ 6652 0	FACTORED MAXIMU GROSS REACTION GROSS VERT HORZ DOWN 6652 0 7752	FACTORED MAXIMUM FACT GROSS REACTION GROSS REACTIO VERT HORZ DOWN HORZ 6652 0 7752 0	FACTORED         MAXIMUM FACTORED           GROSS REACTION         GROSS REACTION           VERT         HORZ         DOWN         HORZ         UPLIFT           6652         0         7752         0         -2564	FACTORED         MAXIMUM FACTORED         INPUT           GROSS REACTION         GROSS REACTION         BRG           VERT         HORZ         DOWN         HORZ         UPLIT IN-SX           6652         0         -7752         0         -2564         2-4			

PROVIDE ANCHORAGE AT BEARING JOINT 5 FOR 2564 LBS FACTORED PROVIDE ANCHORAGE AT BEARING JOINT 8 FOR 2465 LBS FACTORED

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES

PROVIDE FOR 383 LBS FACTORED HORIZONTAL REACTION AT JOINT 8

UNF	UNFACTORED REACTIONS							
	1ST LCASE	MAX./I	MIN. COMPON	ENT REACTI	ONS			
JT	COMBINE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
5	4668	3267 / 0	0/0	0/0	2749 / -2732	1401 / 0	0/0	
8	4660	3260 / 0	0/0	0/0	2717 / -2661	1400 / 0	0/0	
HOF 8	RIZONTAL RI 	EACTIONS 0/0	0/0	0/0	274 / -181	0/0	0 /0	
8		0/0	0/0	0/0	274 / -181	0/0	0 /0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 5, 8 BEARING SIZE FACTOR = 1.08 AT JNT(S) 5 ( BASED ON SUPPORT DEPTH = 1-8 )

<u>BRACING</u> TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.32 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x4 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 4-5. DBS = 6-0-0. CBF = 182 LBS. 1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 4-6. DBS = 4-0-0 . CBF = 74 LBS.

 $\label{eq:dbs} DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) TO EACH PLY USING (0.122°X3°) SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.$ 

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING TOTAL LOAD CASES: (11)

СН	CHORDS WEBS								
	K. FACTORED	FACTO	RED			***	MAX. FACTO	RED	
MEMB.	FORCE	VERT. LC	OAD LC	1 MAX	MAX.	MEMB	. FORCE	MAX	
	(LBS)	(PI	_F) (	CSI (LC)	UNBRAC	;	(LBS)	CSI (LC)	
FR-TO		FROM	TO		LENGTH	FR-TO	)		
1- 2	-7540 / 2432	-77.3	-77.3	0.19 (7)	4.32	7- 2	-966 / 3264	0.21 (3)	
2-3	-4764 / 1639	-77.3	-77.3	0.15 (7)	5.21	2-6	-3215 / 1221	0.61(3)	
3- 4	-3991 / 1447	-77.3	-77.3	0.16 (7)	5.51	6-3	-723 / 2293	0.20 (5)	
5- 4	-5595 / 1937	0.0	0.0	0.36(1)	4.96	6- 4	-2234 / 6775	0.44(3)	
8- 1	-5750 / 1887	0.0	0.0	0.17 (4)	6.13	1- 7	-1970 / 6423	0.42(4)	
8- 7	-364 / 245	-875.6	-875.6	0.20(3)	6.25				
7-6	-2107 / 6329	-875.6	-875.6	0.33(3)	6.25				
6- 5	-169 / 176	-875.6	-875 6	0.24(3)	6.25				

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK
COELEMENTS, BOOK BY THE WIND FORCE RESISTING SYSTEM).INTERNAL TEGORY 2). BUILDING MAY BE LOCATED ON TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

RECEIVED TOWN OF MILTON MAR 29, 2017

17-4978 **BUILDING DIVISION** 

# **DESIGN CRITERIA**

SPECIFIED LOADS: LL = DL = LL = DL = AD = CH. PSF 3.0 PSF 7.0 PSF TOTAL LOAD 33.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

GIRDER TYPE: CStdGirder START DISTANCE = 0-0 START SPAN CARRIED = 38-4-8 END DISTANCE = 14-3-4 END SPAN CARRIED = 38-4-8 END WALL WIDTH = 2-4 APPLIED TO FRONT SIDE OF BOTTOM CHORD. - ADDT'L LOADS BASED ON 55 % OF GSL

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012, BCBC 2012, ABC 2014 CSA 086-09 - TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.46")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.46") CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.36 (4-5:1) , BC=0.33 (6-7:3) , WB=0.61 (2-6:3) , SSI=0.53 (5-6:4)

COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN MAX MIN

MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (7) (INPUT = 0.90 ) JSI METAL= 0.64 (1) (INPUT = 1.00) JOB NAME TRUSS NAME QUANTITY PLY 3 TW0317-048 GIRD11

JOB DESC. TR-GREENPARK-LECCO RIDGE-BLOCK 327 TRUSS DESC.

DRWG NO.

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Kott Lumber Uxbridge, Stouffville, ON, TW

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 PLATES (table is in inches)

 JT TYPE
 PLATES
 W
 LEN
 Y
 X

 7
 BMWW+t
 MT20
 5.0
 8.0
 4.00
 2.00

 8
 BMV1+p
 MT20
 2.0
 4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

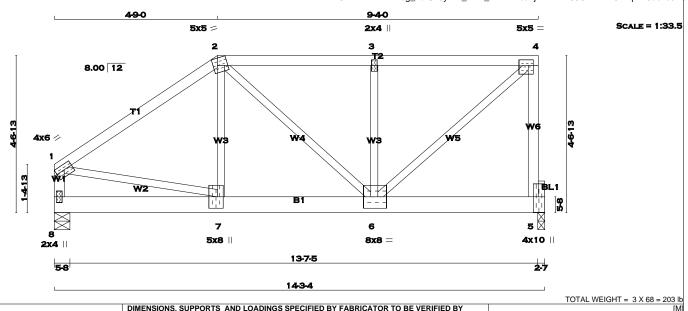
JOB PESC TR-GREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY PLY DRWG NO. 3 TW0317-048 **GIRD12** 

Kott Lumber Uxbridge, Stouffville, ON, TW

Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:24 2017 Page 1 ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-wzr\_xWPkBbazjm??IPxl85CkETA78l?fq?M8aozcJK5

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TW0317-048



LUMBER								
N. L. G. A. RULES								
CHORDS	SIZE		LUMBER	DESCR.				
1 - 2	2x4	DRY	No.2	SPF				
2 - 4	2x4	DRY	No.2	SPF				
5 - 4	2x4	DRY	No.2	SPF				
8 - 1	2x4	DRY	No.2	SPF				
8 - 5	2x6	DRY	2100F 1.8E	SPF				
BEARING B	LOCKS							
BL1	2x6	DRY	No.2	SPF				
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF				

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF  $\underline{\mathbf{3}}$  TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORL	S #ROWS	SURFACE	LOAD(PLF)
		SPACING (IN)	
TOP CH	HORDS: (0.1	22"X3") SPIRAL NAILS	
1-2	1	12	TOP
2-4	1	12	TOP
4-5	1	12	TOP
8- 1	1	12	TOP
вотто	M CHORDS	: (0.122"X3") SPIRAL NAIL	.S
8- 5	2	` 5	SIDE(607.5)
WEBS :	(0.122"X3")	SPIRAL NAILS	- ()
	,		

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

<u>FL</u>	FLATES (table is ill littles)								
JT	TYPE	PLATES	· W	LEN	Y X				
1	TMVW-t	MT20	4.0	6.0	1.75 Edge				
2	TTWW-m	MT20	5.0	5.0	2.50 1.50				
3	TMW+w	MT20	2.0	4.0					
4	TMVW-t	MT20	5.0	5.0	2.00 1.75				
5	BMVK1+t	MT20	4.0	10.0	Edge 1.75				
6	BMWWW-t	MT20	8.0	8.0	4.00 2.50				



DIMENSIONS, SUPPORTS	AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY
BUILDING DESIGNER	

BEAI	RINGS						
	FACTOR	ED	MAXIMUN	/ FACTO	ORED	INPUT	REQRD
	<b>GROSS RE</b>	ACTION	GROSS F	REACTIO	N	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	<b>UPLIFT</b>	IN-SX	IN-SX
5	6084	0	7062	0	-2328	2-7	2-7
8	6073	0	7058	233	-2290	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 5 FOR 2328 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 8 FOR 2290 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES

PROVIDE FOR 233 LBS FACTORED HORIZONTAL REACTION AT JOINT 8

UNFACTORED REACTIONS

	151 LUASE		MIN. COMPONE	ENT REACTI	ONS		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
5	4270	2988 / 0	0/0	0/0	2446 / -2486	1281 / 0	0/0
8	4262	2981 / 0	0/0	0/0	2464 / -2459	1281 / 0	0/0
HOR 8	IZONTAL RE	EACTIONS 0/0	0/0	0/0	167 / -117	0/0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 5, 8 BEARING SIZE FACTOR = 1.09 AT JNT(S) 5 ( BASED ON SUPPORT DEPTH = 1-8 )

<u>BRACING</u> TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.46 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (11)

	C H O R D S WE B S MAX. FACTORED FACTORED MAX. FACTORED							
MEMB.	FORCE	VERT. LC	AD LC	1 MAX	MAX.	MEMB	. FORCE	MAX
	(LBS)	(PI	_F) (	CSI (LC)	UNBRAC	)	(LBS)	CSI (LC)
FR-TO		FROM	TO		LENGTH	FR-TO	)	
1- 2	-6829 / 2229	-77.3	-77.3	0.21 (7)	4.46	7- 2	-1041 / 3579	0.23 (4)
2-3	-5607 / 1937	-77.3	-77.3	0.15 (7)	4.89	2-6	-234 / 103	0.05(3)
3- 4	-5608 / 1938	-77.3	-77.3	0.15 (7)	4.88	6-3	-423 / 348	0.04(1)
5- 4	-5254 / 1803	0.0	0.0	0.46 (1)	6.36	6- 4	-2554 / 7706	0.54 (7)
8- 1	-5169 / 1743	0.0	0.0	0.15 (4)	6.40	1- 7	-1764 / 5767	0.38 (3)
8- 7 7- 6	-214 / 154 -1847 / 5777	-794.1	-794.1	0.20 (3) 0.27 (3)				
6- 5	-213 / 145	-794.1	-794.1	0.19 (3)	6.25			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF ( 9.0) PSF AT WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (\$3.0) PSP AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

**RECEIVED** TOWN OF MILTON MAR 29, 2017 17-4978 **BUILDING DIVISION** 

#### **DESIGN CRITERIA**

SPECIFIED LOADS:							
TOP	CH.	LL	=	23.3	PSF		
		DL	=	3.0	PSF		
BOT	CH.	LL	=	0.0	PSF		
		DL	=	7.0	PSF		
TOTA	L LO	AD	=	33.3	PSF		

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

GIRDER TYPE: CStdGirder START DISTANCE = 0-0 START SPAN CARRIED = 35-2-8 END DISTANCE = 14-3-4 END SPAN CARRIED = 35-2-8 END WALL WIDTH = 5-8 APPLIED TO FRONT SIDE OF BOTTOM CHORD. - ADDT'L LOADS BASED ON 55 % OF GSL

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012 . BCBC 2012 . ABC 2014 CSA 086-09

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.46") CALCULATED VERT. DEFL.(LL)= L/ 999 (0.06") ALLOWABLE DEFL.(TL)= L/360 (0.46") CALCULATED VERT. DEFL.(TL)= L/ 999 (0.09")

CSI: TC=0.46 (4-5:1) , BC=0.27 (6-7:3) , WB=0.54 (4-6:7) , SSI=0.46 (7-8:4)

COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN MAX MIN

MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (2) (INPUT = 0.90 JSI METAL= 0.58 (1) (INPUT = 1.00) JOB NAME TRUSS NAME QUANTITY PLY 3 TW0317-048 GIRD12

JOB DESC. TR-GREENPARK-LECCO RIDGE-BLOCK 327 TRUSS DESC.

DRWG NO.

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Kott Lumber Uxbridge, Stouffville, ON, TW

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 PLATES (table is in inches)

 JT TYPE
 PLATES
 W
 LEN
 Y
 X

 7
 BMWW+t
 MT20
 5.0
 8.0
 4.00
 2.00

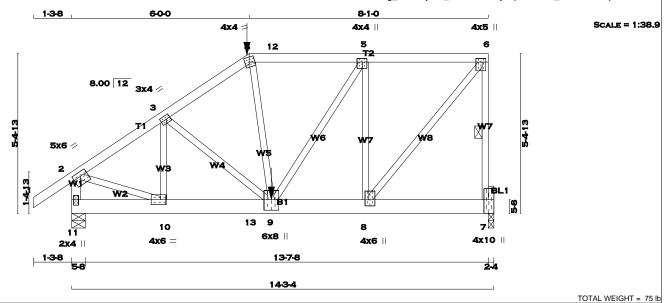
 8
 BMV1+p
 MT20
 2.0
 4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Kott Lumber Uxbridge, Stouffville, ON, TW Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:25 2017 Page 1 ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-O9PM8rPMyviqKwaBr6S\_hJluwsY6t8tp3f5i6EzcJK4



LUMBER								
	N. L. G. A. RULES							
CHORDS	SIZE		LUMBER	DESCR.				
1 - 4	2x4	DRY	No.2	SPF				
4 - 6	2x4	DRY	No.2	SPF				
7 - 6	2x3	DRY	No.2	SPF				
11 - 2	2x4	DRY	No.2	SPF				
11 - 7	2x6	DRY	2100F 1.8E	SPF				
BEARING B								
BL1	2x6	DRY	No.2	SPF				
ALL WEBS	2x3	DRY	No.2	SPF				
EXCEPT								

DRY: SEASONED LUMBER.

PLATES (table is in inches)										
JT	TYPE	PLATES	W	LEN	Y X					
2	TMVW-t	MT20	5.0	6.0	1.75 3.00					
3	TMWW-t	MT20	3.0	4.0	1.50 1.50					
4	TTW-m	MT20	4.0	4.0	2.00 1.75					
5	TMWW+t	MT20	4.0	4.0	1.50 1.75					
6	TMVW+p	MT20	4.0	5.0	1.50 1.50					
7	BMVK1+t	MT20	4.0	10.0	Edge 1.75					
8	BMWW+t	MT20	4.0	6.0	2.50 1.50					
9	BMWWW+t	MT20	6.0	8.0	4.25 3.00					
10	BMWW-t	MT20	4.0	6.0	2.00 2.25					
11	BMV1+p	MT20	2.0	4.0	2.25 1.00					

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED NAME OF THE PROPERTY OF THE PR DELEGATED TO THE BUILDING DESIGNER



DIMENSIONS, SUPPORTS BUILDING DESIGNER	AND LOADINGS	SPECIFIED	BY FABRI	CATOR TO BE	VERIFIED BY
BEARINGS					
FACTORED	MAXIMUM FAC	TORED INF	PUT R	EQRD	

	FACTO	RED	MAXIMU	M FACT	INPUT	REQRD	
	GROSS R	EACTION	GROSS REACTION			BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
7	1721	0	1968	0	-968	2-4	2-4
11	1979	0	2260	304	-1011	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 7 FOR 968 LBS\_FACTORED\_UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 11 FOR 1011 LBS\_FACTORED\_UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES

PROVIDE FOR 304 LBS FACTORED HORIZONTAL REACTION AT JOINT 11

UNFACTORED REACTIONS

	1ST LCASE	MAX./	MIN. COMPON	<u>IENT REACTIO</u>	ONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL			
7	1206	852 / 0	0/0	0/0	617 / -919	354 / 0	0/0			
11	1389	970 / 0	0/0	0/0	702 / -992	420 / 0	0/0			
HOR	HORIZONTAL REACTIONS									
11		0/0	0/0	0/0	217 / -145	0/0	0 /0			

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 7, 11 BEARING SIZE FACTOR = 1.08 AT JNT(S) 7 ( BASED ON SUPPORT DEPTH = 1-8 )

<u>BRACING</u> TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.17 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 6-7. DBS = 8-0-0 . CBF = 84 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL	TOTAL LOAD CASES: (11)									
	ORDS X. FACTORED	FACTORE	D		WE	BS MAX. FACTO	)RED			
MEMB.		VERT. LOAD	LC1 MAX		MEMB.		MAX CSI (LC)			
FR-TO		FROM TO			H FR-TO		()			
1- 2	0/29	-77.3 -	77.3 0.11 (	1) 10.00	10- 3	-492 / 343	0.09(3)			
2-3	-2404 / 1131		77.3 0.29 (			-142 / 140	0.05 (5)			
3-4	-2585 / 1330		77.3 0.33 (			-220 / 695	0.16(1)			
4-12	-2251 / 1189	-145.8 -14	45.8 0.34 (	7) 4.32	9- 5	-708 / 1507	0.41 (7)			
12-5	-2251 / 1189	-77.3 -	77.3 0.34 (	7) 4.32	8- 5	-1677 / 912	0.67 (3)			
5-6	-1450 / 814	-77.3 -	77.3 0.28 (	7) 5.19	8-6	-1156 / 2338	0.82 (7)			
7-6	-1937 / 991	0.0	0.0 0.53 (	7) 4.20	2-10	-858 / 2089	0.45 (1)			
11-2	-2202 / 1030	0.0	0.0 0.21 (	1) 5.75						
11-10	-285 / 194	-33.0 -	33.0 0.02 (	11) 6.25						
10-13	-961 / 2039	-33.0 -	33.0 0.15 (	3) 6.25						
13-9	-961 / 2039	-33.0 -	33.0 0.15 (	3) 6.25						
9-8	-616 / 1446	-17.5 -	17.5 0.13 (	3) 6.25						
8- 7	-74 / 1 <u>19</u>	-17.5 -	17.5 0.02 (	11) 6.25						
FACT	RF ON	R/ ED DA								
JT	ALC .	IA			DIR.	TYPE				
	11 -0-	4			ERT	TOTAL	ECEIVE	٦.		
9	-01 -75	-2000	926	RONT V	ERT					
						TOW	N OF MILT	O		

COE IS AN INTEGRAL PART OF THIS DRAWING AS IT WIN CONTAINS SPECIFICATIONS AND CRITERIA USE CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

ING SYSTEM : HOPERNAL MAY BE LOCATED ON EABITHLED HINGS BAY WASHON

### **DESIGN CRITERIA**

\*\*\* SPECIAL LOADS ANALYSIS \*\*\*
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS: LL = 23.3 DL = 3.0 LL = 0.0 TOP CH. PSF PSF PSF BOT CH. DL PSF PSF TOTAL LOAD 33.3

#### SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

GIRDER TYPE: CPrimeHip LEFT SETBACK = 6-0-0 RIGHT SETBACK = 0-0 END SETBACK = 6-0-0 END WALL WIDTH = 5-8 CORNER FRAMING TYPE: CONVENTIONAL END JACK TYPE: CONVENTIONAL APPLIED TO FRONT SIDE ADDT'L LOADS BASED ON 55 % OF GSL LOADS APPLIED TO FIRST 6-9-0 OF SPAN MEASURED FROM THE LEFT.

\*\*\* NON STANDARD GIRDER \*\*\* ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH: PART 9 OF OBC 2012, BCBC 2012, ABC 2014 - CSA 086-09 - TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.47") CALCULATED VERT. DEFL.(LL)= L/999 (0.06") ALLOWABLE DEFL.(TL)= L/360 (0.47") CALCULATED VERT. DEFL.(TL)= L/999 (0.09")

CSI: TC=0.53 (6-7:7), BC=0.15 (9-10:3), WB=0.82 (6-8:7), SSI=0.17 (5-6:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

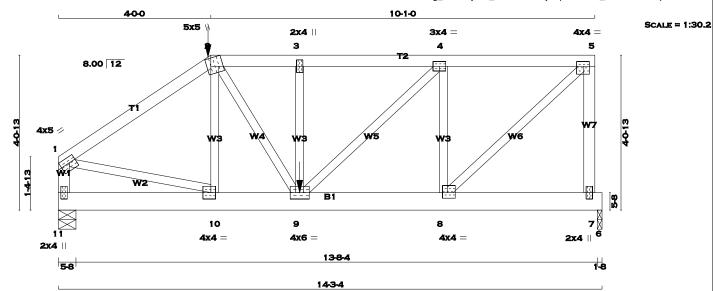
NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

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LUMBER				
N. L. G. A. R	ULES			
CHORDS	SIZE		LUMBER	DESCR.
1 - 2	2x4	DRY	No.2	SPF
2 - 5	2x4	DRY	No.2	SPF
7 - 5	2x4	DRY	No.2	SPF
11 - 1	2x4	DRY	No.2	SPF
11 - 6	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER

PL/	PLATES (table is in inches)										
JT	TYPE	PLATES	W	LEN	Υ :	X					
1	TMVW-t	MT20	4.0	5.0	1.75	Edge					
2	TTWW+m	MT20	5.0	5.0	Edge :	3.75					
3	TMW+w	MT20	2.0	4.0							
4	TMWW-t	MT20	3.0	4.0							
5	TMVW-t	MT20	4.0	4.0	1.50	1.75					
7	BMV+p	MT20	2.0	4.0							
8	BMWW-t	MT20	4.0	4.0	1.75	1.50					
9	BMWWW-t	MT20	4.0	6.0							
10	BMWW-t	MT20	4.0	4.0	2.00	1.50					
11	BMV1+n	MT20	2.0	4.0							

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 168.5 lbs FACTORED DOWN AND 117.0
lbs FACTORED UP AT 4-0-0 ON TOP CHORD,
AND 570.1 lbs FACTORED DOWN AND 256.3 lbs FACTORED UP AT 6-4-0 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY **BUILDING DESIGNER** 

BEA	BEARINGS										
	FACTORED		MAXIMUM FACTORED			INPUT	REQRD				
	GROSS RE	EACTION	GROSS	REACTIO	N	BRG	BRG				
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX				
11	1117	0	1252	206	-587	5-8	5-8				
6	949	0	1042	0	-521	1-8	1-8				

PROVIDE ANCHORAGE AT BEARING JOINT 11 FOR 587 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 6 FOR 521 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES

MAX./MIN. COMPONENT REACTIONS
SNOW LIVE PERM.LIVE WIND

0/0

DEAD

237 / 0

338 / -572

232 / -500

SOIL

0/0

PROVIDE FOR 206 LBS FACTORED HORIZONTAL REACTION AT JOINT 11

0/0

ge '5		
'5		
0		
0		

HORIZONTAL REACTIONS 0/0 0/0 147 / -105 0/0 0 /0 0/0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 11, 6

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.24 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

11 6

LOADING TOTAL LOAD CASES: (11)

UNFACTORED REACTIONS

547 / 0

1ST LCASE COMBINED

СН	ORDS		WEBS					
MAX	(. FACTORED	FACTO	RED				MAX. FACTO	RED
MEMB.	FORCE	VERT. LC	DAD LC	1 MAX	MAX.	MEMB.	FORCE	MAX
	(LBS)	(Pl	_F) (	CSI (LC)	<b>UNBRAC</b>		(LBS)	CSI (LC)
FR-TO		FROM	TO		LENGTH	FR-TO		
1-2	-1368 / 683	-77.3	-77.3	0.35 (7)	5.24	10- 2	-137 / 156	0.03 (7)
2-3	-1419 / 784	-107.2	-107.2	0.18 (7)	5.40	2-9	-274 / 510	0.12(1)
3- 4	-1420 / 785	-77.3	-77.3	0.25 (7)	5.28	9-3	-292 / 226	0.07 (1)
4- 5	-1012 / 581	-77.3	-77.3	0.22 (7)	6.01	9- 4	-285 / 570	0.13 (7)
7- 5	-1068 / 572	0.0	0.0	0.24(1)	7.63	8- 4	-764 / 481	0.18 (3)
11-1	-1208 / 616	0.0	0.0	0.12(1)	7.29	8- 5	-694 / 1379	0.31(1)
						1-10	-470 / 1146	0.26 (1)
11-10	-186 / 137			0.05 (11)	6.25			
10-9	-510 / 1144	-24.2	-24.2	0.19(1)	6.25			
9-8	-436 / 1010	-17.5	-17.5	0.18 (1)	6.25			
8- 7	-28 / 72	-17.5	-17.5	0.13(1)	6.25			
7-6	0/0	-17.5	-17.5	0.13(1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
2	4-0-0	-129	-169	117	FRONT	VERT	TOTAL
9	6-4-0	-486	-570	256	FRONT	VERT	TOTAL

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CPCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

**RECEIVED** TOWN OF MILTON MAR 29, 2017 17-4978 **BUILDING DIVISION** 

### **DESIGN CRITERIA**

\*\*\* SPECIAL LOADS ANALYSIS \*\*\*
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE

TOTAL WEIGHT = 70 lb

SPECIFIED LOADS: LL = 23.3 DL = 3.0 LL = 0.0 TOP CH. PSF PSF PSF BOT CH. DL PSF TOTAL LOAD 33.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

GIRDER TYPE: CPrimeHip LEFT SETBACK = 4-0-0 RIGHT SETBACK = 0-0 END SETBACK = 4-0-0 END WALL WIDTH = 5-8 CORNER FRAMING TYPE: CONVENTIONAL END JACK TYPE: CONVENTIONAL APPLIED TO FRONT SIDE ADDT'L LOADS BASED ON 55 % OF GSL LOADS APPLIED TO FIRST 6-4-0 OF SPAN MEASURED FROM THE LEFT.

\*\*\* NON STANDARD GIRDER \*\*\* ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH: PART 9 OF OBC 2012, BCBC 2012, ABC 2014 - CSA 086-09 - TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.48") CALCULATED VERT. DEFL.(LL)= L/999 (0.03") ALLOWABLE DEFL.(TL)= L/360 (0.48") CALCULATED VERT. DEFL.(TL)= L/999 (0.06")

CSI: TC=0.35 (1-2:7), BC=0.19 (9-10:1), WB=0.31 (5-8:1), SSI=0.63 (6-7:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2



JOB PESC TR-GREENPARK-LECCO RIDGE-BLOCK 327 Page 27 of 159 TW0317-048 TW0317-048 **GIRD15** Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:25 2017 Page Kott Lumber Uxbridge, Stouffville, ON, TW ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-O9PM8rPMyviqKwaBr6S\_hJlvNsSEtCnp3f5i6EzcJK4 1-3-8 4-1-13 1487 41-12 1-3-8 SCALE = 1:40.0 6x8 \ 3x5 || 2x4 || 4x4 = 6x8 < 5 4 6 FT: 8.00 12 4x16 = 4x8 < 9 \_ B2 B1 ⊠ 10 12 16 15 14 13 11 4x6 = 4x5 || 5x8 = 4x6 = 4x6 = 4x6 = 2x4 || 1-3-8 22-1-0 1-3-8 5-8 23-0-0 TOTAL WEIGHT = 112 lb LUMBER N. L. G. A. RULES CHORDS SIZE DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER **DESIGN CRITERIA** LUMBER DESCR BEARINGS FACTORED \*\*\* SPECIAL LOADS ANALYSIS \*\*\* GEOMETRY AND/OR BASIC LOADS CHANGED 3 DRY No.2 SPF SPF MAXIMUM FACTORED INPLIT REQRD No.2 GROSS REACTION GROSS REACTION HORZ UPLIFT IN-SX 9 2 8 2x4 DRY No 2 VFRT HOR7 DOWN IN-SX BY USER LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE . 17 -2x6 No.2 2378 203 -1273 5-8 10-2x6 DRY No.2 SPF 10 -983 5-8 5-8 12 No.2 PROVIDE ANCHORAGE AT BEARING JOINT 17 FOR 1273 LBS FACTORED SPECIFIED LOADS: 12 -10 No.2 LL = DL = LL = TOP CH. 23.3 PSF ALL WEBS EXCEPT 3.0 PSF PSF NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES
SHALL BE PROVIDED BY BUILDG. DESIGNER BOT CH. DL PSF DRY: SEASONED LUMBER. TOTAL LOAD 33.3 PROVIDE FOR 203 LBS FACTORED HORIZONTAL REACTION AT JOINT 17 SPACING = 24.0 IN. C/C UNFACTORED REACTIONS 1ST LCASE COMBINED MAX./MIN. COMPONENT REACTIONS
SNOW LIVE PERM.LIVE WIND PLATES (table is in inches) DEAD LOADING IN FLAT SECTION BASED ON A SLOPE SOIL TYPE TMVW-p 792 / -1228 382 / -941 0/0 PLATES W IFN Y 1668 1172 / 0 0/0 0/0 495 / 0 MT20 4.0 16.0 Edge 5.50 10 GIRDER TYPE: CPrimeHip TTWW+m MT20 6.0 8.0 Edge 2.00 3.0 2.0 4.0 TMWW+1 MT20 1.75 1.50 HORIZONTAL REACTIONS 17 --- 0/0 LEFT SETBACK = 4-1-13 RIGHT SETBACK = 4-1-12 ΓMW+w 0/0 0/0 145 / -145 0/0 0 /0 MT20 4.0 END SETBACK = 6-0-0 END WALL WIDTH = 5-8 CORNER FRAMING TYPE: CONVENTIONAL TMWW-t MT20 1 75 1 75 6.0 4.0 2.00 3.25 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 17, 10 MT20 1.50 3.75 TMVW-t MT20 8.0 MT20 MT20 2.0 4.0 4.0 4.0 6.0 END JACK TYPE: CONVENTIONAL APPLIED TO FRONT SIDE 10 BMV1+r 2.25 1.00 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.06 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 5.95 FT. OR RIGID CEILING DIRECTLY BMWW-t 2.00 2.25 ADDT'L LOADS BASED ON 55 % OF GSL BS-t MT20 6.0 BMWW-t 6.0 LOADS APPLIED TO FIRST 9-11-4 OF SPAN 13 14 BMWWW-t 2.75 4.00 MEASURED FROM THE LEFT. MT20 5.0 8.0 4.0 4.0 5.0 6.0 BMWW+t MT20 2.25 ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED. MT20 NON STANDARD GIRDER \*\*\* BMWW-t 2.00 1.50 BMV1+p <u>LOADING</u> TOTAL LOAD CASES: (11) 3.0 4.0 ADDT'L USER-DEFINED LOADS APPLIED TO Edge - INDICATES REFERENCE CORNER OF PLATE THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010 TOUCHES EDGE OF CHORD. CHORDS MAX. FACTORED WEBS MAX. FACTORED FORCE MAX FACTORED A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED. MEMB. FORCE VERT. LOAD LC1 MAX MAX. MEMB. (PLF) FROM TO CSI (LC) UNBRAC FR-TO LENGTH FR-TO THIS DESIGN COMPLIES WITH: 1-2 2-3 0/29 -77.3 -77.3 -77.3 0.11 (1) -77.3 0.50 (7) 10.00 16- 3 3-15 -472 / 372 - PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09 HANGERS NOTES -3115 / 1537 -950 / 1874 3.62 0.41(1) SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 314.8 lbs FACTORED DOWN AND 218.5 3- 4 4- 5 -3729 / 1912 -145.8 -145.8 0.42 (7) -145.8 -145.8 0.46 (7) 3 41 15-4 -1347 / 780 0.32 (3) -4345 / 2177 -481 / 1016 3.12 5- 6 6- 7 7- 8 (55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. -77.3 -77.3 0.50 (7) -4345 / 2177 3.06 14- 5 -390/2940.09(3)Ibs FACTORED UP AT 4-1-13 ON TOP CHORD, AND 1213.8 lbs FACTORED DOWN AND 545.6 lbs FACTORED UP AT 9-11-4 ON BOTTOM CHORD. 14- 6 13- 6 13- 7 11- 7 -3419 / 1759 -2270 / 1128 -77.3 -77.3 -77.3 -77.3 0.44 (8) 0.43 (8) 3.52 4.28 -585 / 1217 -1195 / 696 0.33 (7) 0.29 (3) RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD 8- 9 17- 2 -77.3 0.11 (1) 0.0 0.17 (1) 0/29 -77.3 10.00 -1018 / 2023 0.55 (7) ALLOWABLE DEFL.(LL)= L/360 (0.77") CALCULATED VERT. DEFL.(LL) = L/999 (0.15") ALLOWABLE DEFL.(TL)= L/360 (0.77") CALCULATED VERT. DEFL.(TL) = L/999 (0.24") DESIGN FOR UNSPECIFIED CONNECTION(S) IS -2628 / 1311 6.44 -360 / 246 0.09 (3) 2-16 -1172 / 2631 DELEGATED TO THE BUILDING DESIGNER. 10-8 -1963 / 1004 0.0 0.13(1) 7.20 0.57(1)0.43 (1) -822 / 1945 17-16 -184 / 192 -33.0 -33.0 0.07 (11) 6.25 -33.0 -33.0 -17.5 16-15 -1183 / 2598 -33.0 0.31 (3) -33.0 0.53 (3) 6.25 15-14 CSI: TC=0.50 (5-6:7) , BC=0.53 (14-15:3) 14-13 13-12 12-11 -17.5 0.50 (1) -1551 / 3428 6.20 WB=0.57 (2-16:1) , SSI=0.22 (3-4:3) T.L. WISE 100083566 -774 / 1884 -774 / 1884 -17.5 -17.5 -17.5 0.24 (1) -17.5 0.24 (1) 6.25 6.25 DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 11-10 -9/20-17.5 -17.5 0.04 (11) 10.00 COMP=1.00 SHEAR=1.00 TENS= 1.00 FACTORED CONCENTRATED LOADS (LBS) COMPANION LIVE LOAD FACTOR = 0.50 LC1 -241 MAX+ 219 .IT MAX-FACE DIR TYPE LOC -315 4-1-13 FRONT VERT TOTAL 14 546 FRONT VERT TOTAL TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE 100083566 TRUSS MANUFACTURING PLANT. ERENCE VELOCITY PRESSURE OF ( 9.0) PSF AT /E GRADE AND USING EXTERNAL PEAK I WIND FORCE RESIS ING SYSTEM INTERNAL TEGORY 2), BUILDING MAY BRUDGATER VISION OM R HT AE HE {M WIN NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN WCE OF ONTAR (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT EASTOU/WHITONESMIALVIAON 618 354 1667 822 2284 1656 READ ALL NOTES ON THIS PAGE AND ON THE MAR 29, 2017 ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE PLATE PLACEMENT TOL. = 0.250 inches March 10, 2017 IS AN INTEGRAL PART OF THIS DRAWING AS IT 17-4978

CONTAINS SPECIFICATIONS AND CRITERIA USED

IN THE DESIGN OF THIS COMPONENT.

JOB NAME

TRUSS NAME

QUANTITY

CONTINUED ON PAGE 2

PLATE ROTATION TOL. = 5.0 Deg.

**BUILDING DIVISION** 

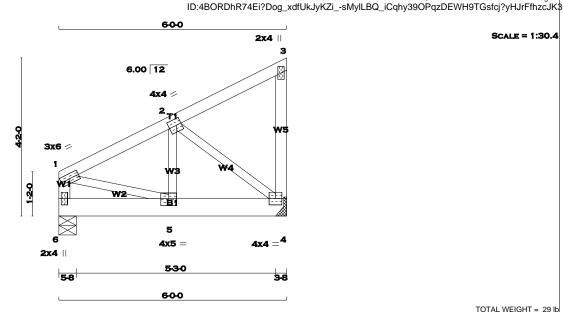
DRWG NO

JOB PESC TR-GREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY PLY TRUSS DESC TW0317-048 **GIRD16** 

Kott Lumber Uxbridge, Stouffville, ON, TW

DRWG NO. Page 28 of 159 TW0317-048

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TOTAL WEIGHT = 29 lb

SCALE = 1:30.4

LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR 3 2x4 2x4 DRY No.2 No.2 SPF SPF DRY 2x4 DRY No 2 SPF 2x6 No.2 ALL WEBS DRY No.2 SPF 2x3 **EXCEPT** 

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Υ	X
1	TMVW-t	MT20	3.0	6.0		
2	TMWW-t	MT20	4.0	4.0	2.00	1.50
3	TMV+p	MT20	2.0	4.0		
4	BMVW1-t	MT20	4.0	4.0		
5	BMWW-t	MT20	4.0	5.0	2.25	2.50
6	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS

	FACTO	RED	MAXIMU	M FACT	ORED	INPUT	REQRD
	GROSS RI	EACTION	GROSS REACTION			BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
4	1034	0	1187	0	-454	HANGER	BY OTHERS
						MIN. SEAT	T SIZE: 3-8
6	1034	0	1186	242	-389	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 4 FOR 454 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 6 FOR 389 LBS FACTORED UPLIFT

PROVIDE FOR 242 LBS FACTORED HORIZONTAL REACTION AT JOINT 6

UNFACTORED REACTIONS

	1ST LCASE	MAX./	MIN. COMPO	NENT REACTION	ONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
4	726	508 / 0	0/0	0/0	381 / -465	218 / 0	0/0		
6	726	508 / 0	0/0	0/0	379 / -418	218 / 0	0/0		
HOF	HORIZONTAL REACTIONS								
6		0/0	0/0	0/0	173 / -70	0/0	0 /0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 6

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.10 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (11)

СН	ORDS					W E	BS		
MAX	(. FACTORED	FACTO	RED				MAX. FACTO	DRED	
MEMB.	FORCE	VERT. LC	DAD LC	1 MAX	MAX.	MEMB	. FORCE	MAX	
	(LBS)	(PI	LF) (	CSI (LC)	UNBRAC		(LBS)	CSI (LC)	
FR-TO		FROM	TO		LENGTH	FR-TO			
1-2	-1022 / 337	-77.3	-77.3	0.16 (7)	6.10	5- 2	-231 / 855	0.18 (3)	
2-3	-72 / 71	-77.3	-77.3	0.11 (7)	6.25	2- 4	-1147 / 491	0.26(3)	
4-3	-100 / 78	0.0	0.0	0.10 (7)	7.81	1- 5	-258 / 955	0.21 (4)	
6- 1	-792 / 287	0.0	0.0	0.08 (1)	7.81				
6- 5	-226 / 91	267.5	267.5	0.16 (3)	6.25				
5- 4	-369 / 915			0.16 (3)	6.25				

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF ( 9.0) PSE AT WIND LOAD APPLIED IS DERIVED FROM REPERSING VELOCITY FRESSURE OF (3.9), 25 AT (40-0.0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.

#### **DESIGN CRITERIA**

SPECIFIED LOADS: CH. PSF 3.0 PSF

LL = DL = LL = DL = AD = PSF 7.0 PSF TOTAL LOAD 33.3 PSF

### SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder START DISTANCE = 0-0 START SPAN CARRIED = 13-0-0 END DISTANCE = 6-0-0 END SPAN CARRIED = 13-0-0 END WALL WIDTH = 5-8 APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09

- TPIC 2011

(55 % OF 27 2 P.S.F. G.S.I. PLUS 8 4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20\*) CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01\*) ALLOWABLE DEFL.(TL)= L/360 (0.20\*) CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02\*)

CSI: TC=0.16 (1-2:7) , BC=0.26 (4-5:3) , WB=0.26 (2-4:3) , SSI=0.31 (5-6:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN MAX MIN

MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (4) (INPUT = 0.90) JSI METAL= 0.34 (4) (INPUT = 1.00)





READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

TOWN OF MILTON MAR 29, 2017 17-4978 **BUILDING DIVISION** 

**RECEIVED** 

JOB NAME TW0317-048

TRUSS NAME **GIRD17**  QUANTITY

JOB PESC PREENPARK-LECCO RIDGE-BLOCK 327 TRUSS DESC

DRWG NO

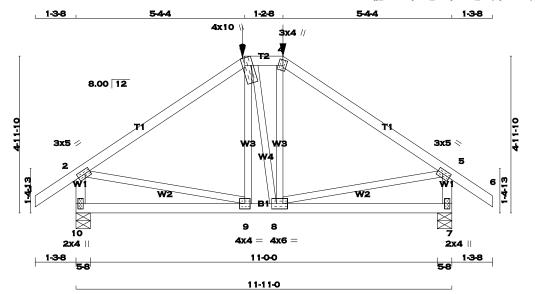
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SCALE = 1:36.5

TOTAL WEIGHT = 55 lb

Kott Lumber Uxbridge, Stouffville, ON, TW

Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:26 2017 Page 1 ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-sMylLBQ\_iCqhy39OPqzDEWH4BGt\_clXyHJrFfhzcJK3



LUMBER N. L. G. A. CHORDS LUMBER DESCR SIZE 3 2x4 DRY No.2 No.2 SPF SPF 2x4 6 2x4 DRY No 2 SPF . 10 -2x4 No.2 5 2x4 DRY No.2 SPF 10 -No.2 DRY ALL WERS 2x3 DRY No.2 SPF EXCEPT

DRY: SEASONED LUMBER

PLATES (table is in inches)
JT TYPE PLATES

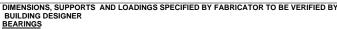
LEN Y 5.0 1.50 2.00 10.0 Edge 1.00 4.0 2.00 1.25 TMVW-t MT20 TTWW+m 4.0 5.0 4.0 TTW+m MT20 3.0 TMVW-t MT20 3.0 MT20 RMWWW-t MT20 4.0 6.0 BMWW-t 4.0 10 BMV1+p MT20 2.0 4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 225.5 lbs FACTORED DOWN AND 156.6
lbs FACTORED UP AT 6-6-12, AND 225.5 lbs
FACTORED DOWN AND 156.6 lbs FACTORED UP
AT 5-4-4 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.



	KIII						
	FACTO	RED	MAXIMUM FACTORED			INPUT	REQRD
	GROSS REACTION GROSS REACTION					BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
10	902	0	999	-239	-450	5-8	5-8
7	902	0	996	0	-450	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 10 FOR 450 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 7 FOR 450 LBS FACTORED UPLIFT

PROVIDE FOR 239 LBS FACTORED HORIZONTAL REACTION AT JOINT 10

UNFACTORED REACTIONS

	1ST LCASE	MAX./I	MIN. COMPO	NENT REACTION	ONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL			
10	630	456 / 0	0/0	0/0	243 / -434	174 / 0	0/0			
7	630	456 / 0	0/0	0/0	235 / -434	174 / 0	0/0			
HOF	HORIZONTAL REACTIONS									
10		0/0	0/0	0/0	171 / -171	0/0	0 /0			

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 10, 7

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.94 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

<u>LOADING</u> TOTAL LOAD CASES: (11)

CHORDS WEBS											
MAX.	FACTORED	FACTO	RED				MAX. FACTO	ORED			
MEMB.	FORCE	VERT. LC	AD LC	1 MAX	MAX.	MEMB.	FORCE	MAX			
	(LBS)	(PL	_F) (	CSI (LC)	<b>UNBRAC</b>	0	(LBS)	CSI (LC)			
FR-TO		FROM	TO		LENGTH	FR-TO					
1- 2	0 / 29	-77.3	-77.3	0.11(1)	10.00	9- 3	-52 / 104	0.02 (11)			
2-3	-851 / 447	-77.3	-77.3	0.50 (7)	5.94	3-8	-76 / 93	0.03 (5)			
3- 4	-711 / 454	-107.2	-107.2	0.09 (8)	6.25	8- 4	-65 / 165	0.04 (5)			
4- 5	-858 / 448	-77.3	-77.3	0.50 (8)	5.96	2-9	-257 / 730	0.16 (4)			
5-6	0 / 29	-77.3	-77.3	0.11(1)	10.00	8- 5	-262 / 735	0.16(3)			
10-2	-945 / 488	0.0	0.0	0.10(1)	7.81						
7- 5	-940 / 488	0.0	0.0	0.10(1)	7.81						
10-9	-218 / 229	-24.2	-24.2	0.18 (11)	6.25						
9-8	-285 / 724	-24.2	-24.2	0.24 (11)	6.25						
8- 7	-10 / 21	-24.2	-24.2	0.19 (11)	10.00						
FACTOR		TRATED I (	74 DS (1	BS)							

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
3	5-4-4	-173	-226	157	FRONT	VERT	TOTAL
4	6-6-12	-173	-226	157	FRONT	VERT	TOTAL

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM],INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

**RECEIVED** TOWN OF MILTON MAR 29, 2017 17-4978 **BUILDING DIVISION** 

### **DESIGN CRITERIA**

SPECIFIED LOADS: LL = DL = LL = DL = AD = CH. PSF 3.0 PSF PSF 7.0 PSF TOTAL LOAD 33.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

GIRDER TYPE: CPrimeHip SIDE SETBACK = 5-4-4 END SETBACK = 4-0-0 END WALL WIDTH = 5-8 CORNER FRAMING TYPE: CONVENTIONAL END JACK TYPE: CONVENTIONAL APPLIED TO FRONT SIDE - ADDT'L LOADS BASED ON 55 % OF GSL

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014 - CSA 086-09 - TPIC 2011

THIS DESIGN COMPLIES WITH:

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.40") CALCULATED VERT. DEFL.(LL)= L/ 999 (0.01") ALLOWABLE DEFL.(TL)= L/360 (0.40") CALCULATED VERT. DEFL.(TL)= L/999 (0.04")

CSI: TC=0.50 (4-5:8) , BC=0.24 (8-9:11) , WB=0.16 (5-8:3) , SSI=0.15 (2-3:7)

COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

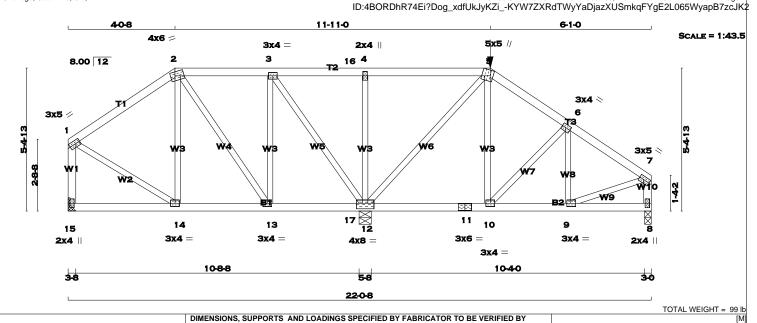
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (8) (INPUT = 0.90 JSI METAL= 0.31 (5) (INPUT = 1.00)

QUANTITY POBPES GREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME PLY DRWG NO. Page 30 of 159 TW0317-048 TW0317-048 GIRD18 Kott Lumber Uxbridge, Stouffville, ON, TW Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:27 2017 Page 1



LUMBER				
N. L. G. A. R	ULES			
CHORDS	SIZE		LUMBER	DESCR.
1 - 2	2x4	DRY	No.2	SPF
2 - 5	2x4	DRY	No.2	SPF
5 - 7	2x4	DRY	No.2	SPF
15 - 1	2x4	DRY	No.2	SPF
8 - 7	2x4	DRY	No.2	SPF
15 - 11	2x4	DRY	No.2	SPF
11 - 8	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER

PL/	PLATES (table is in inches)									
JT	TYPE	PLATES	W	LEN	Υ	X				
1	TMVW-t	MT20	3.0	5.0	1.50	2.00				
2	TTWW-m	MT20	4.0	6.0	1.75	2.00				
3	TMWW-t	MT20	3.0	4.0						
4	TMW+w	MT20	2.0	4.0						
5	TTWW+m	MT20	5.0	5.0	Edge	3.75				
6	TMWW-t	MT20	3.0	4.0	1.50	1.50				
7	TMVW-t	MT20	3.0	5.0	1.50	2.00				
8	BMV1+p	MT20	2.0	4.0						
9	BMWW-t	MT20	3.0	4.0	1.50	1.75				
10,	13, 14									
10	BMWW-t	MT20	3.0	4.0						
11	BS-t	MT20	3.0	6.0						
12	BMWWW1-t	MT20	4.0	8.0	2.50	4.00				
15	BMV1+p	MT20	2.0	4.0						

- INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 461.3 lbs FACTORED DOWN AND 320.2
lbs FACTORED UP AT 15-11-8 ON TOP CHORD.
DESIGN FOR UNSPECIFIED CONNECTION(S) IS
DELEGATED TO THE BUILDING DESIGNER.



DIMENSIONS, SUPPORTS	AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY
BUILDING DESIGNER	

BEA	<u>EARINGS</u>										
	FACTO	RED	MAXIMUM FACTORED			INPUT	REQRD				
	GROSS R	ACTION GROSS REACTION E				BRG	BRG				
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX				
15	454	0	480	-248	-212	HANGER E	BY OTHERS				
						MIN. SEAT	SIZE: 3-8				
12	1820	0	2041	0	-1039	5-8	5-8				
8	747	0	850	0	-341	3-0	3-0				

PROVIDE ANCHORAGE AT BEARING JOINT 15 FOR 212 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 12 FOR 1039 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 8 FOR 341 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 248 LBS FACTORED HORIZONTAL REACTION AT JOINT 15

UNF	UNFACTORED REACTIONS										
	1ST LCASE	MAX./	MIN. COMPO	NENT REACTION	ONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL				
15	319	219/0	0/0	0/0	65 / -216	100 / 0	0/0				
12	1274	909 / 0	0/0	0/0	552 / -977	365 / 0	0/0				
8	529	345 / 0	0/0	0/0	257 / -362	184 / 0	0/0				
HOR	IZONTAL RE	ACTIONS									
15		0/0	0/0	0/0	154 / -177	0/0	0 /0				
BEA	RING MATER	IAL TO BE S	SPF NO.2 OR	BETTER AT JO	DINT(S) 8						

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (11)

СН	ORDS					WE	BS		
MAX	. FACTORED	FACTO	RED				MAX. FACTO	RED	
MEMB.	FORCE	VERT. LC	DAD LC	1 MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PI	LF) (	CSI (LC)	<b>UNBRAC</b>	)	(LBS)	CSI (LC)	
FR-TO		FROM	TO		LENGTH	FR-TO		, ,	
1-2	-283 / 180	-77.3	-77.3	0.28 (7)	6.25	14- 2	-42 / 71	0.02(1)	
2-3	-165 / 195	-77.3	-77.3	0.15(1)	6.25	2-13	-145 / 79	0.10(3)	
3-16	-4 / 107	-77.3	-77.3	0.45 (3)	10.00	13-3	-27 / 153	0.03(3)	
16- 4	-4 / 107	-149.9	-149.9	0.45(3)	10.00	3-12	-473 / 260	0.32(3)	
4- 5	-5 / 108	-149.9	-149.9	0.46 (3)	10.00	12-4	-769 / 561	0.33(3)	
5-6	-732 / 456	-77.3	-77.3	0.20 (8)	6.25	12-5	-1037 / 547	0.91 (4)	
6- 7	-833 / 389	-77.3	-77.3	0.19 (8)	6.25	10- 5	-15 / 266	0.08 (11)	
15- 1	-451 / 232	0.0	0.0	0.07 (7)	7.81	10-6	-195 / 165	0.06 (4)	
8- 7	-803 / 372	0.0	0.0	0.08(1)	7.81	9-6	-117 / 157	0.03 (8)	
						1-14	-30 / 249	0.06(1)	
15-14	-167 / 226	-17.5	-17.5	0.08 (11)	6.25	9- 7	-260 / 723	0.16(1)	
14-13	-130 / 251	-17.5	-17.5	0.10 (11	6.25				
13-17	-89 / 201	-17.5	-17.5	0.15 (11)	6.25				
17-12	-89 / 201	-33.9	-33.9	0.15 (11)	6.25				
12-11	-203 / 580	-33.9	-33.9	0.19(1)	6.25				
11-10	-203 / 580	-33.9	-33.9	0.19(1)	6.25				
10-9	-227 / 679	-33.9	-33.9	0.17 (11)	6.25				
9-8	-9 / 20	-33.9	-33.9	0.08 (11)	10.00				
FACTOR	RED CONCENT	TRATED I	OADS (	RS)					
JT	OF CONCER	TO THE DEC			CE [	DIR.	TYPE		
51			200			JIN.	TOTAL		

RESSURED ELIVED WIND ERENCE VELOCITY F {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING COE WIN READ ALL NOTES ON THIS PAGE AND ON THE

ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

EXTERWAN POFK MILTON ING SYSTEM).INTERNAL MAY BIS INTERNAL EAST (0-0) FT-IN-SX AWAY 17-4978 **BUILDING DIVISION** 

### **DESIGN CRITERIA**

SPECIFIED LOADS: LL = DL = LL = DL = AD = CH. 3.0 PSF 7.0 PSF TOTAL LOAD 33.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

GIRDER TYPE: CPrimeHip LEFT SETBACK = 4-0-8 RIGHT SETBACK = 6-1-0 END SETBACK = 6-0-0 END WALL WIDTH = 3-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL LOADS APPLIED TO FIRST 11-5-8 OF SPAN MEASURED FROM THE RIGHT.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09 - TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.36") CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01") ALLOWABLE DEFL.(TL)= L/360 (0.36") CALCULATED VERT. DEFL.(TL)= L/ 999 (0.03")

CSI: TC=0.46 (4-5:3) , BC=0.19 (10-12:1) , WB=0.91 (5-12:4) , SSI=0.36 (4-5:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (9) (INPUT = 0.90) JSI METAL= 0.27 (7) (INPUT = 1.00) JOB NAME TRUSS NAME

TW0317-048

QUANTITY **GIRD19** 

PLY

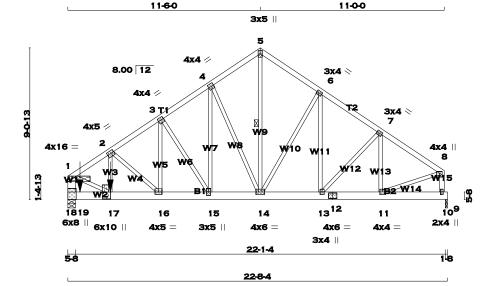
JOB PESC TR-GREENPARK-LECCO RIDGE-BLOCK 327 TRUSS DESC

DRWG NO.

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SCALE = 1:68.8

Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:27 2017 Page 1 Kott Lumber Uxbridge, Stouffville, ON, TW ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-KYW7ZXRdTWyYaDjazXUSmkqlCg9QL0r5WyapB7zcJK2



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR 5 8 2x4 DRY No.2 No.2 SPF SPF DRY 2x4 18 -2x6 DRY No 2 SPF 10 -2x4 No.2 18 -12 2x6 DRY No.2 SPF 12 -9 No.2 SPF DRY ALL WERS 2x3 DRY No.2 SPF EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF  $\underline{\mathbf{2}}$  TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS		SURFACE	LOAD(PLF)						
		SPACING (IN)							
TOP CH	TOP CHORDS : (0.122"X3") SPIRAL NAILS								
1-5	1	12	TOP						
5-8	1	12	TOP						
10-8	1	12	TOP						
18- 1	2	12	TOP						
BOTTON	A CHORDS	S: (0.122"X3") SPI	RAL NAILS						
18- 12	2	12	SIDE(0.0)						
12-9	2	12	TOP						
WEBS:	WEBS: (0.122"X3") SPIRAL NAILS								
2x3	1	6							

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING.
REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP

PL	ATES	(table	is	in	inches)	
IT	TVDE		J	1 4	TEC	1

JT	TYPE	PLATES	W	LEN	Υ	X	
1	TMVW-p	MT20	4.0	16.0	0.50	Edge	
2	TMWW-t	MT20	4.0	5.0	1.75	1.25	
3	TMWW-t	MT20	4.0	4.0	1.75	1.00	
4	TMWW-t	MT20	4.0	4.0	2.00	1.25	
5	TT\//±n	MT20	3.0	5.0			



#### DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY **BUILDING DESIGNER**

3EA	RINGS						
	FACTOR	ED	MAXIMU	M FACTO	ORED	INPUT	REQRD
	GROSS RE	ACTION	GROSS F	REACTIO	N	BRG	BRG
JΤ	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
18	6489	0	7496	365	-3307	5-8	5-8
9	1655	0	1800	0	-750	1-8	1-8

PROVIDE ANCHORAGE AT BEARING JOINT 18 FOR 3307 LBS FACTORED PROVIDE ANCHORAGE AT BEARING JOINT 9 FOR 750 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES

PROVIDE FOR 365 LBS FACTORED HORIZONTAL REACTION AT JOINT 18

### UNFACTORED REACTIONS

	1ST LCASE	: <u>MAX./</u>	MIN. COMPON	<u>ENTREACT</u>	ONS		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
18	4554	3186 / 0	0/0	0/0	2519 / -3242	1368 / 0	0/0
9	1161	811/0	0/0	0/0	365 / -761	351 / 0	0/0
HOR 18	IZONTAL RE	EACTIONS 0/0	0/0	0/0	261 / -255	0/0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 18, 9

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.60 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 5-14. DBS = 16-0-0 . CBF = 90 LBS.

DBS = DIAGONAL BRACE SPACING (MAX), CBF = CUMULATIVE BRACING FORCE, FASTEN LATERAL BRACE(S) TO EACH PLY USING (0.122"X3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING TOTAL LOAD CASES: (11)

		` '							
C F	HORDS					WE	BS		
MA	X. FACTORED	FACTO	RED				MAX. FACTO	RED	
MEMB		VERT. LC		1 MAX	MAX.	MEMB		MAX	
	(LBS)	(Pl	_F) (	CSI (LC)	UNBRAG	0	(LBS)	CSI (LC)	
FR-TO		FROM	TO		LENGTH	FR-TO		, ,	
1- 2	-7210 / 3199	-77.3	-77.3	0.29 (7)	3.60	17- 2	-1603 / 3590	0.39(3)	
2-3	-4296 / 1936	-77.3	-77.3	0.19 (7)	4.58	2-16	-3257 / 1579	0.43 (3)	
3- 4	-2868 / 1354	-77.3	-77.3	0.15 (7)	5.41	16-3	-1109 / 2427	0.26 (3)	
4- 5	-2013 / 1061	-77.3	-77.3	0.13 (7)	6.18	3-15	-2283 / 1164	0.57 (3)	
5- 6	-2026 / 1056	-77.3	-77.3	0.15 (8)	6.16	15- 4	-884 / 1855	0.36 (7)	
6- 7	-2127 / 999	-77.3	-77.3	0.16 (8)	6.04	4-14	-1813 / 1003	0.92 (3)	
7-8	-2053 / 887	-77.3	-77.3	0.15 (8)	6.12	14- 5	-1031 / 2000	0.26 (7)	
18- 1	-6464 / 2863	0.0	0.0	0.20(1)	5.92	14- 6	-274 / 310	0.13 (4)	
10-8	-1869 / 813	0.0	0.0	0.10(1)	7.81	13-6	-76 / 138	0.03(3)	
						13-7	-107 / 137	0.02 (6)	
18-19	-346 / 348	-17.5	-17.5	0.24(3)	6.25	11-7	-398 / 225	0.05 (3)	
19-17	-346 / 348	-17.5	-17.5	0.24(3)	6.25	1-17	-2780 / 6417	0.69 (3)	
17-16	-2770 / 6068	-17.5	-17.5	0.48 (3)	6.25	11-8	-681 / 1823	0.21 (1)	
16-15	-1597 / 3649	-17.5	-17.5	0.24 (3)	6.25			` '	
15-14	-971 / 2422	-17.5	-17.5	0.18(1)	6.25				
14-13	61/ / 1			(	6.25				
13-12	/ 1	17 17 17	- 5	0.1 1)	6.25				
12-11	_ / 1	17	- 5	0.1 1)	6.25				
11-10	-1 / 2	17	- 5	0.1 1)	6.25			-OEN/E	$\overline{}$
10-9	0/0		5	0.1 - (1)	10.00		R	ECEIVE	ט
				( )			TOW	N OF MIL	TC
FACT	DE 4 D 4 1 1 NO	TEO 011 T			D 011 T1		<b>7</b>   ' ' ' '	O. WILL	. ~
JT	READ ALL NO						YPE MA	AR 29, 201	17
17	ENGINEERING	NOTE P	AGE EI	NP-1. TI	HE NOT	E PAGE	TOTAL		

IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED

IN THE DESIGN OF THIS COMPONENT.

NC

DTAL 17-4978 **BUILDING DIVISION** 

# **DESIGN CRITERIA**

\*\*\* SPECIAL LOADS ANALYSIS \*\*\*
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER

TOTAL WEIGHT = 2 X 127 = 254 lb

LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS: LL = DL = LL = TOP CH. 23.3 PSF PSF PSF 3.0 BOT CH. DL PSF TOTAL LOAD 33.3 PSF

### SPACING = 24.0 IN. C/C

\*\*\* NON STANDARD GIRDER \*\*\*
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012, BCBC 2012, ABC 2014 - CSA 086-09

- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.76")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.76") CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.29 (1-2:7), BC=0.48 (16-17:3), WB=0.92 (4-14:3), SSI=0.55 (9-10:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (7) (INPUT = 0.90 ) JSI METAL= 0.65 (17) (INPUT = 1.00 )

POBPES GREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY PLY TRUSS DESC. 2 TW0317-048 GIRD19

Kott Lumber Uxbridge, Stouffville, ON, TW

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DRWG NO.

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TW0317-048

 
 PLATES
 (table is in inches)

 JT
 TYPE
 PLATES
 W

 6
 TMWW-t
 MT20
 3.0
 LEN Y X 4.0 1.50 1.50 4.0 1.50 1.50 4.0 1.25 2.00 4.0 2.00 1.75 6.0 4.0 6.0 5.0 6 TMWW-t TMVW+p MT20 3.0 4.0 2.0 4.0 4.0 3.0 4.0 3.0 4.0 6.0 MT20 10 BMV+n MT20 BMWW-t MT20 BS-t MT20 MT20 MT20 MT20 MT20 BMWW+t BMWWW-t 5.0 5.0 1.75 2.50 10.0 5.00 1.50 BMWW+t BMWW-t BMWW+t MT20 MT20 18 BMV1+t MT20 6.0 8.0 5.50

 $\ensuremath{\mathsf{Edge}}$  - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 1172.0 Ibs FACTORED DOWN AND 526.8
Ibs FACTORED UP AT 8-12, AND 5875.2 Ibs
FACTORED DOWN AND 2640.9 Ibs FACTORED
UP AT 2-6-8 ON BOTTOM CHORD. DESIGN FOR
UNSPECIFIED CONNECTION(S) IS DELEGATED
TO THE BUILDING DESIGNER.

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM].INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE

17-4978

**BUILDING DIVISION** 

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IN THE DESIGN OF THIS COMPONENT.

CONTAINS SPECIFICATIONS AND CRITERIA USED

DRWG NO

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JOB DE SC TR-GREENPARK-LECCO RIDGE-BLOCK 327

JOB NAME

TRUSS NAME

March 10, 2017

QUANTITY

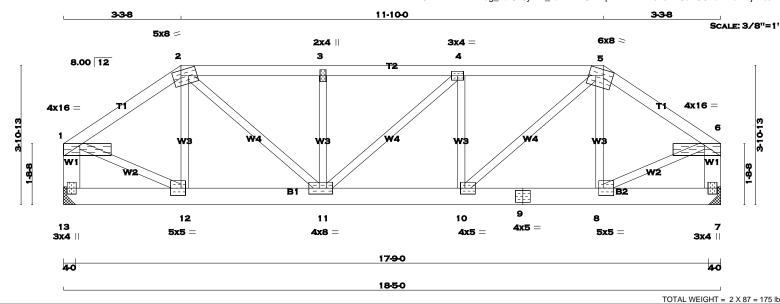
QUANTITY

PLY

2

Kott Lumber Uxbridge, Stouffville, ON, TW

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LUMBER				
N. L. G. A. R	RULES			
CHORDS	SIZE		LUMBER	DESCR.
1 - 2	2x4	DRY	No.2	SPF
2 - 5	2x4	DRY	No.2	SPF
5 - 6	2x4	DRY	No.2	SPF
13 - 1	2x6	DRY	No.2	SPF
7 - 6	2x6	DRY	No.2	SPF
13 - 9	2x6	DRY	No.2	SPF
9 - 7	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF <u>2</u> TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS

CHORDS #ROWS		LOAD(PLF)
	SPACING (IN)	
TOP CHORDS: (0.	.122"X3") SPIŘAĹ N	AILS
1-2 1	12	TOP
2-5 1	12	TOP
5-6 1	12	TOP
13-1 2	12	TOP
7-6 2	12	TOP
BOTTOM CHORDS	S: (0.122"X3") SPIR	AL NAILS
13-9 2	12	SIDE(248.2)
9-7 2	12	SIDE(248.2)
WEBS: (0.122"X3"	) SPIRAL NAILS	
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DI ATES (table is in inches)

PL/	PLATES (table is in inches)										
JT	TYPE	PLATES	W	LEN	Υ	X					
1	TMVW-p	MT20	4.0	16.0	0.50	Edge					
2	TTWW-m	MT20	5.0	8.0	1.75	2.75					
3	TMW+w	MT20	2.0	4.0							
4	TMWW-t	MT20	3.0	4.0							
5	TTWW-m	MT20	6.0	8.0	2.00	3.25					
6	TMVW-p	MT20	4.0	16.0	Edge	5.50					



#### DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY **BUILDING DESIGNER**

DEA	BEARINGS										
	FACTO	RED	MAXIMUM FACTORED			INPUT	REQRD				
	GROSS R	GROSS	REACTIO	N	BRG	BRG					
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX				
13	4770	0	5496	-162	-1855	HANGER I	BY OTHERS				
						MIN. SEAT	SIZE: 4-0				
7	4770	0	5474	0	-1855	HANGER I	BY OTHERS				
						MIN. SEAT	SIZE: 4-0				

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES. SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 162 LBS FACTORED HORIZONTAL REACTION AT JOINT 13

UNFACTORED REACTIONS

	1ST LCASE	MAX.	MIN. COMPON	IENT REACT	ONS		
JT	COMBINED	) SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
13	3347	2342 / 0	0/0	0/0	1816 / -1972	1006 / 0	0/0
7	3347	2342 / 0	0/0	0/0	1761 / -1972	1006 / 0	0/0
HOR 13	IZONTAL RE	EACTIONS 0/0	0/0	0/0	115 / -115	0/0	0 /0

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.63 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (11)

CHORDS					WEBS			
MAX	K. FACTORED	FACTORED			MAX. FACTORED			
MEMB.	FORCE	VERT. LC	DAD LC	1 MAX	MAX.	MEME	<ol><li>FORCE</li></ol>	MAX
	(LBS)	(PI	LF) (	CSI (LC)	<b>UNBRAC</b>		(LBS)	CSI (LC)
FR-TO		FROM	TO		LENGTH	FR-TC	)	
1- 2	-5339 / 1832	-77.3	-77.3	0.22 (7)	4.14	12- 2	-124 / 428	0.05 (4)
2-3	-6995 / 2463	-77.3	-77.3	0.25(7)	3.63	2-11	-1199 / 3384	0.37(1)
3- 4	-6995 / 2464	-77.3	-77.3	0.24 (7)	3.67	11- 3	-308 / 254	0.03(1)
4- 5	-7004 / 2472	-77.3	-77.3	0.25 (8)	3.63	11- 4	-34 / 45	0.01 (4)
5-6	-5321 / 1830	-77.3	-77.3	0.22(8)	4.16	10- 4	-308 / 266	0.03(1)
13- 1	-4811 / 1669	0.0		0.15(1)		10- 5	-1213 / 3435	0.37(3)
7-6	-4785 / 1667	0.0	0.0	0.16 (3)	6.68	8- 5	-132 / 414	0.04 (4)
						1-12	-1529 / 4724	0.51 (1)
13-12	-136 / 150	-440.6	-440.6	0.21 (3)	6.25	8- 6	-1527 / 4732	0.51 (3)
12-11	-1454 / 4459	-440.6	-440.6	0.46(3)	6.25			
11-10	-2311 / 7006	-440.6	-440.6	0.56(3)	6.25			
10-9	-1402 / 4430	-440.6	-440.6	0.45(3)	6.25			
9- 8	-1402 / 4430			0.45(3)	6.25			
8- 7	-12 / 26	-440.6	-440.6	0.20(3)	6.25			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

**RECEIVED** TOWN OF MILTON MAR 29, 2017 17-4978 **BUILDING DIVISION** 

### **DESIGN CRITERIA**

SPECIFIED LOADS: LL = DL = LL = DL = AD = CH. PSF 3.0 PSF PSF 7.0 PSF TOTAL LOAD 33.3 PSF

#### SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

GIRDER TYPE: CStdGirder START DISTANCE = 5-8 START SPAN CARRIED = 19-11-10 END DISTANCE = 18-10-8 END SPAN CARRIED = 19-11-10 END WALL WIDTH = 1-8 APPLIED TO FRONT SIDE OF BOTTOM CHORD. - ADDT'L LOADS BASED ON 55 % OF GSL

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012, BCBC 2012, ABC 2014 - CSA 086-09 - TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61\*) CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10\*) ALLOWABLE DEFL.(TL)= L/360 (0.61\*) CALCULATED VERT. DEFL.(TL)= L/ 999 (0.16\*)

CSI: TC=0.25 (2-3:7) , BC=0.56 (10-11:3) , WB=0.51 (1-12:1) , SSI=0.31 (12-13:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (2) (INPUT = 0.90) JSI METAL= 0.68 (9) (INPUT = 1.00)

JOB DESC TR-GREENPARK-LECCO RIDGE-BLOCK 327 TRUSS DESC. JOB NAME TRUSS NAME QUANTITY PLY 2 TW0317-048 GIRD21

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Kott Lumber Uxbridge, Stouffville, ON, TW

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

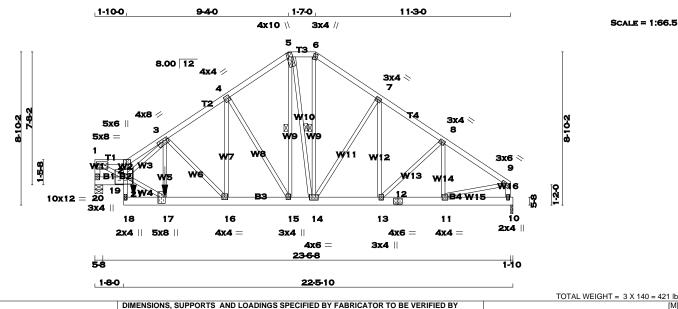


READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

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DRWG NO



LUMBER				
N. L. G. A. R	ULES			
CHORDS	SIZE		LUMBER	DESCR.
20 - 1	2x4	DRY	No.2	SPF
1 - 2	2x4	DRY	No.2	SPF
2 - 5	2x4	DRY	No.2	SPF
5 - 6	2x4	DRY	No.2	SPF
6 - 9	2x4	DRY	No.2	SPF
10 - 9	2x4	DRY	No.2	SPF
20 - 19	2x6	DRY	No.2	SPF
18 - 2	2x3	DRY	No.2	SPF
18 - 12	2x6	DRY	No.2	SPF
12 - 10	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER

DESIGN CONSISTS OF <u>3</u> TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS

CHORDS	S #ROWS		LOAD(PLF)
		SPACING (	
TOP CH	ORDS : (0.1	22"X3") SPIR	AL NAILS
20- 1	1	12	TOP
1-2	1	12	TOP
2-5	1	12	TOP
5-6	1	12	TOP
6-9	1	12	TOP
10- 9	1	12	TOP
BOTTON	A CHORDS	: (0.122"X3")	SPIRAL NAILS
20- 19	2	12	SIDE(90.3)
18- 12	3	4	SIDE(1746.3)
12- 10	2	12	TOP
2- 18	1	12	SIDE(42.9)
WEBS:	(0.122"X3")	SPIRAL NAIL	S
2x3	1 ′	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN

GIRDER NAILING ASSUMES NAILED HANGERS ARE

FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLE SHOWN IS THE FOLIVALENT LIDEAPPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP



IMENSIONS, SUPPORTS	AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY
BUILDING DESIGNER	

BEA	RINGS						
	FACTO	RED	MAXIMU	M FACT	ORED	INPUT	REQRD
	GROSS R	EACTION	GROSS	REACTIO	N	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
20	6209	0	7097	441	-3140	5-8	5-8
10	2037	0	2218	0	-976	1-10	1-10

PROVIDE ANCHORAGE AT BEARING JOINT 20 FOR 3140 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 10 FOR 976 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES

PROVIDE FOR 441 LBS FACTORED HORIZONTAL REACTION AT JOINT 20

UNFACTORED REACT	<u> FIONS</u>
10T1010E	****/

	1ST LCASE	MAX./I	<u>MIN. COMPON</u>	<u>NENT REACT</u>	IONS		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
20	4358	3048 / 0	0/0	0/0	2219 / -3084	1309 / 0	0/0
10	1429	1000 / 0	0/0	0/0	453 / -973	429 / 0	0/0
HOR	IZONTAL RE	EACTIONS					
20		0/0	0/0	0/0	315 / -323	0/0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 20, 10

DI

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.26 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 4.05 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 5-15. DBS = 16-0-0 . CBF = 88 LBS. - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 5-14, 6-14. DBS = 20-0-0 . CBF = 74 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) TO EACH PLY USING (0.122"X3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

# **LOADING**

15-14

12-11

-900 / 2419

TOTAL LOAD CASES: (11)

CH	HORDS		WEBS					
MA	X. FACTORED	FACTORED	1			MAX. FACTO	RED	
MEMB	. FORCE	VERT. LOAD	LC1 MAX	MAX.	MEMB	. FORCE	MAX	
	(LBS)	(PLF)	CSI (LC	) UNBRA	С	(LBS)	CSI (LC)	
FR-TO	, , ,	FROM TO		LENGTH	FR-TO	) '		
20- 1	-6961 / 3113	0.0	0.0 0.21 (	1) 5.67	1-19	-5941 / 13388	0.88 (1)	
1- 2	-11434 / 5091	-77.3 -77	7.3 0.25 (7	7) 3.61	19-17	-3271 / 7264	0.47 (3)	
2-3	-13786 / 6209	-77.3 -77	7.3 0.32 (7	7) 3.26	19- 3	-3272 / 7174	0.47 (3)	
3-4	-4628 / 2148	-77.3 -77	7.3 0.15 (7	7) 5.29	17- 3	-1175 / 2390	0.16 (3)	
4- 5	-2877 / 1461	-77.3 -77	7.3 0.11 (7	6.25	3-16	-4184 / 1990	0.56 (3)	
5-6	-2305 / 1234	-77.3 -77	7.3 0.07 (8	6.25	16- 4	-1402 / 3112	0.29 (7)	
6- 7	-2769 / 1392	-77.3 -77	7.3 0.11 (8	3) 6.25	4-15	-2908 / 1487	0.89(3)	
7-8	-2824 / 1347	-77.3 -77	7.3 0.11 (8	3) 6.25	15- 5	-986 / 2022	0.15 (7)	
8- 9	-2697 / 1206	-77.3 -77	7.3 0.11 (8	3) 6.25	5-14	-629 / 342	0.10(3)	
10-9	-2174 / 989	0.0	0.0 (	7.81	14- 6	-661 / 1353	0.10(7)	
					14- 7	-207 / 271	0.06(4)	
20-19	-421 / 200	-134.1 -134	1.1 0.02 (3	3) 6.25	13- 7	-181 / 163	0.04(3)	
18-19	-451 / 1041	0.00	0.0 0.39 (3	7.81	13-8	-108 / 131	0.01 (6)	
19- 2	82 / 3		(	4.05	11-8	-470 / 272	0.03 (3)	
18-21	/ 3	34 -1	1 0.1	6.25	11- 9	-939 / 2322	0.16(1)	
21-17	- / 3	17 -	5 0.1	6.25				
17-16	= 305 / 68 J	17 -	5 0.2	6.25			= O = 1) /F	_

14-13 13-12 READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

-17.5 0.16 (1)

RECEIVED TOWN OF MILTON MAR 29, 2017 17-4978

**BUILDING DIVISION** 

## **DESIGN CRITERIA**

\*\*\* SPECIAL LOADS ANALYSIS \*\*\*
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS: LL = DL = LL = TOP CH. 23.3 PSF PSF PSF 3.0 BOT CH. PSF TOTAL LOAD PSF 33.3

## SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

GIRDER TYPE: CStdGirder START DISTANCE = 0-0 START SPAN CARRIED = 6-11-0 FND DISTANCE = 2-2-12 END SPAN CARRIED = 6-11-0 END WALL WIDTH = 0-0 APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

\*\*\* NON STANDARD GIRDER \*\*\* ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF **PART 9. NBCC 2010** 

THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012, BCBC 2012, ABC 2014

- CSA 086-09

- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.80°) CALCULATED VERT. DEFL.(LL)= L/999 (0.10°) ALLOWABLE DEFL.(TL)= L/360 (0.80°) CALCULATED VERT. DEFL.(TL)= L/999 (0.16°)

CSI: TC=0.32 (2-3:7) , BC=0.44 (2-19:3) , WB=0.89 (4-15:3) , SSI=0.18 (17-18:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (16) (INPUT = 0.90 JSI METAL= 0.71 (1) (INPUT = 1.00) JOB NAME TRUSS NAME QUANTITY PLY 3 TW0317-048 **GIRD22** 

POBPES GREENPARK-LECCO RIDGE-BLOCK 327 TRUSS DESC.

DRWG NO.

Page 37 of 159 TW0317-048

Kott Lumber Uxbridge, Stouffville, ON, TW

Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:24:05 2017 Page 2 ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-yUhXJ44M8VQckjOTkJPZcisYiYZBQdKBetOLqUzcJGe

 
 PLATES (table is in inches)

 JT TYPE PLATES W

 1 TMVW-t MT20 5.0
 LEN Y X 8.0 2.00 2.50 8.0 2.00 2.50 6.0 Edge 2.25 8.0 1.50 3.75 4.0 2.00 1.00 10.0 Edge 1.02 4.0 2.00 1.50 4.0 1.50 1.50 4.0 4.0 4.0 4.0 6.0 TTV+p TMWWW-t MT20 5.0 4.0 4.0 3.0 3.0 3.0 3.0 4.0 4.0 3.0 4.0 5.0 2.0 10.0 MT20 TMWW-t MT20 MT20 TTW+m MT20 MT20 MT20 MT20 MT20 TMWW-t TMWW-t TMVW-t BMV1+p BMWW-t MT20 BS-t BMWW+t MT20 MT20 12 13 14 15 6.0 4.0 6.0 4.0 4.0 8.0 4.0 12.0 BMWWW-t MT20 1.75 1.50 1.75 2.00 4.25 1.50 BMWW+t MT20 MT20 MT20 MT20 BMWW-t BMWW+t BMV+p BVMWWW-I MT20 BMV1+p 3.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 1155.3 lbs FACTORED DOWN AND 524.6
lbs FACTORED UP AT 2-2-12, AND 5476.7 lbs
FACTORED DOWN AND 2486.8 lbs FACTORED
UP AT 4-0-8 ON BOTTOM CHORD. DESIGN FOR
UNSPECIFIED CONNECTION(S) IS DELEGATED
TO THE BUILDING DESIGNED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS) MAX--5477 -1155 JT 17 21 LOC. 4-0-8 MAX+ 2487 FACE DIR. TYPE LC1 -4715 FRONT FRONT VERT VERT TOTAL TOTAL 2-2-12 -995 525

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.0) PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

JOB PESC PREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY PLY DRWG NO. TRUSS DESC TW0317-048 **GIRD23** 

Kott Lumber Uxbridge, Stouffville, ON, TW

Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:29 2017 Page 1 ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-Hxet\_DSt?7CGpXtz4yXwr9viEUxbp8NO\_G3vF0zcJK0

2 8.00 12 3x4 / 28-13 W3 1413 **B**1 3 2x4 4x4 5-8 200

SCALE = 1:19.3

TOTAL WEIGHT = 11 lb

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TW0317-048

LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR 2x4 DRY No.2 No.2 SPF SPF 2x4 DRY 2x4 DRY No 2 SPF 2x6 No.2 ALL WEBS 2x3 DRY DRY: SEASONED LUMBER. SPF No.2

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Υ	X
1	TMVW-t	MT20	3.0	4.0	1.50	1.00
2	TMV+p	MT20	2.0	4.0		
3	BMVW1-t	MT20	4.0	4.0		
4	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH

TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
BEARINGS

	1111100						
	FACTO	RED	MAXIMU	IM FACT	ORED	INPUT	REQRD
	GROSS R	EACTION	GROSS	REACTIO	N	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
4	222	0	267	138	-86	5-8	5-8
3	222	0	271	0	-151	HANGER	BY OTHERS
						MIN. SEAT	Γ SIZE: 1-12

PROVIDE ANCHORAGE AT BEARING JOINT 4 FOR 150 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 3 FOR 151 LBS FACTORED UPLIFT

PROVIDE FOR 138 LBS FACTORED HORIZONTAL REACTION AT JOINT 4

UNF	UNFACTORED REACTIONS									
	1ST LCASE	MAX./N	IIN. COMPO	NENT REACTION	ONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL			
4	156	109/0	0/0	0/0	111 / -91	47 / 0	0/0			
3	156	109/0	0/0	0/0	121 / -138	47 / 0	0/0			
HOF	RIZONTAL RE	ACTIONS								

0/0

98 / -76

0/0

0 /0

0/0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 4

0/0

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (11)

CHC	RDS	WEBS							
MAX.	FACTORED	FACTO	RED				MAX. FACTO	DRED	
MEMB.	FORCE	VERT. LC	DAD LC	1 MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PI	LF) (	CSI (LC)	UNBRA	С	(LBS)	CSI (LC)	
FR-TO		FROM	TO		LENGTH	FR-TO			
4- 1	-100 / 52	0.0	0.0	0.01 (8)	7.81	1- 3	-57 / 108	0.02 (6)	
1- 2	-59 / 53	-77.3	-77.3	0.06(3)	6.25				
3- 2	-92 / 69	0.0	0.0	0.04 (7)	7.81				
4-3	-118 / 97	-145 0	-145 O	0.06 (3)	6 25				

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

# **DESIGN CRITERIA**

SPECIFIED LOADS: LL = DL = LL = DL = AD = CH. PSF 3.0 PSF PSF

7.0 PSF TOTAL LOAD 33.3 PSF

## SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder START DISTANCE = 0-0 START SPAN CARRIED = 7-4-8 END DISTANCE = 2-0-0 END SPAN CARRIED = 7-4-8 END WALL WIDTH = 0-0

APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09

- TPIC 2011

(55 % OF 27 2 P.S.F. G.S.I. PLUS 8 4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19°) CALCULATED VERT. DEFL.(LL)= L/999 (0.00°) ALLOWABLE DEFL.(TL)= L/360 (0.19°) CALCULATED VERT. DEFL.(TL)= L/999 (0.00°)

CSI: TC=0.06 (1-2:3) , BC=0.06 (3-4:3) , WB=0.02 (1-3:6) , SSI=0.10 (3-4:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.31 (1) (INPUT = 0.90) JSI METAL= 0.04 (3) (INPUT = 1.00)





READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

TOWN OF MILTON MAR 29, 2017 17-4978 **BUILDING DIVISION** 

**RECEIVED** 

JOB PESC TR-GREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY PLY DRWG NO. Page 39 of 159 TW0317-048 TW0317-048 **GIRD26** Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:30 2017 Page 1 Kott Lumber Uxbridge, Stouffville, ON, TW ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-I7CFBZTVmRK7RhS9eg29OMSnVtBJYT3YCwpToSzcJK <u>1-5-</u>0 8-10-0 8-11-0 SCALE = 1:55.7 4x5 = 5x5 // 5 8.00 12 4x4 / 3x4 < R 3x4 / 4x16 = 3 4x5 = 9 wio W 2 W11 81 Bo ြို့ 14 13 10 17 15 12 18 16 2x4 || 3x4 II 5x8 || 3x4 || 3x4 || 4x6 = 4x6 = 4x5 = 3x4 II 1-5-0 5-8 23.7.4 24-2-4 TOTAL WEIGHT = 131 lb DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY LUMBER N. L. G. A. RULES CHORDS SIZE BUILDING DESIGNER **DESIGN CRITERIA** LUMBER DESCR BEARINGS FACTORED 5 7 2x4 DRY No.2 SPF SPF MAXIMUM FACTORED INPLIT REQRD \*\*\* SPECIAL LOADS ANALYSIS \*\*\*
GEOMETRY AND/OR BASIC LOADS CHANGED No.2 GROSS REACTION GROSS REACTION BRG HORZ LIPLIFT IN-SX 9 2x4 DRY No 2 SPF VFRT HOR7 DOWN IN-SX BY USER LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE . 19 -2 2x6 No.2 -1422 5-8 11 -2x4 DRY No.2 SPF 10 1261 -595 1-8 1-8 13 No.2 DRY SPF PROVIDE ANCHORAGE AT BEARING JOINT 19 FOR 1422 LBS FACTORED SPECIFIED LOADS: 13 -10 2x6 No.2 LL = DL = LL = TOP CH. 23.3 PSF ALL WEBS EXCEPT 3.0 PSF PSF NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES
SHALL BE PROVIDED BY BUILDG. DESIGNER BOT CH. ĎL PSF DRY: SEASONED LUMBER. TOTAL LOAD PSF 33.3 PROVIDE FOR 314 LBS FACTORED HORIZONTAL REACTION AT JOINT 19 SPACING = 24.0 IN. C/C UNFACTORED REACTIONS 1ST LCASE COMBINED MAX./MIN. COMPONENT REACTIONS
SNOW LIVE PERM.LIVE WIND PLATES (table is in inches) DEAD SOIL LOADING IN FLAT SECTION BASED ON A SLOPE TYPE TMVW-p 631 / -1387 0/0 PLATES W IFN Y 1972 1394 / 0 0/0 0/0 577 / 0 OF 6.00/12 MT20 4.0 16.0 Edge 5.50 10 183 / -597 1.50 1.50 \*\*\* NON STANDARD GIRDER \*\*\* 3 TMWW-t MT20 3.0 4.0 MT20 4.0 5.0 4.0 TMWW-1 2 00 HORIZONTAL REACTIONS ADDT'L USER-DEFINED LOADS APPLIED TO 4.0 4.0 2.0 5.0 3.0 4.0 2.0 4.0 ALL LOAD CASES. 0/0 0/0 0/0 ΓTWW-m MT20 1.75 1.50 224 / -216 0 /0 19 0/0 TMW+w MT20 5.0 4.0 TTWW+m BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 19, 10 MT20 THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF TMWW-t MT20 1.50 1.50 MT20 MT20 TMVW-p 5.0 1.25 2.50 PART 9, NBCC 2010 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.05 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY BMV+p BMWW-t 4.0 THIS DESIGN COMPLIES WITH 12 MT20 5.0 200 200 BS-t MT20 6.0 - PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09 13 14 BMWW+t 3.0 MT20 4.0 MT20 4.0 3.0 6.0 4.0 BMWWW-ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED. - TPIC 2011 MT20 BMWW+t 1.75 1.50 BMWW+t <u>LOADING</u> TOTAL LOAD CASES: (11) DESIGN ASSUMPTIONS MT20 3.0 4.0 BMWW+1 OVERHANG NOT TO BE ALTERED OR CUT 19 BMV1+p MT20 3.0 4.0 2.25 1.50 CHORDS MAX. FACTORED WEBS Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD. (55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED MAX. FACTORED FACTORED FORCE MEMB FORCE VERT. LOAD LC1 MAX MAX. MEMB. (PLF) FROM TO CSI (LC) UNBRAC ROOF LIVE LOAD A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH FR-TO LENGTH FR-TO ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.81") TEE-LOK TL20 PLATES IS ALLOWED. 1-2 2-3 0/32 -77.3 -77.3 -77.3 0.14 (1) -77.3 0.30 (7) 10.00 17- 4 4-16 -193 / 486 0.11(1) -2729 / 1294 0.47 (3) -794 / 535 4.05 -77.3 -77.3 -77.3 16- 5 5-15 15- 6 3- 4 4- 5 -2197 / 1065 -77.3 0.37 (7) -77.3 0.34 (7) 4 35 -307 / 592 0 27 (7) CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08") -1585 / 838 HANGERS NOTES -116 / 90 0.13 (8) 5- 6 6- 7 7- 8 SPECIAL HANGER(S) OR CONNECTION(S) -77.3 0.21 (8) -1333 / 773 5.48 -296 / 231 0.26(3)REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1863.7 lbs FACTORED DOWN AND 885.3 -1333 / 773 -1442 / 784 -77.3 -77.3 -77.3 0.21 (8) -77.3 0.41 (8) 5.48 5.08 15- 7 14- 7 -252 / 369 -160 / 272 0.27 (7) 0.14 (8) CSI: TC=0.41 (7-8:8), BC=0.41 (17-18:1), WB=0.57 (2-18:1), SSI=0.84 (10-11:1) Ibs\_FACTORED LIP AT 1-10-12 ON BOTTOM 8-9 -1635 / 765 -77.3 -77.3 0.41 (8) 4 82 14-8 -331 / 315 0.20 (4) CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE 19- 2 11- 9 12- 8 2-18 DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00 -2935 / 1377 6.16 0.03 (3) -1355 / 644 0.0 0.0 0.14 (1) 6.99 -1060 / 2507 0.57(1)-549 / 1397 -284 / 545 BUILDING DESIGNER. 12-9 0.33 (1) 19-18 -17.5 -17.5 6.25 COMPANION LIVE LOAD FACTOR = 0.50 -294 / 293 0.15 (3) 18-3 0.12(4)18-17 -1126 / 2297 -17.5 -17.5 -17.5 0.41 (1) 6.25 -515 / 315 0.11 (3) 0.25 (1) 6.25 AUTOSOLVE RIGHT HEEL ONLY TI WISE THE 16-15 -496 / 1329 -17.5 -17.5 0.17 (1) 6.25 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. 15-14 14-13 -379 / 1186 -519 / 1369 -17.5 -17.5 -17.5 0.16 (1) -17.5 0.20 (1) 6.25 6.25 -17.5 -17.5 13-12 -519 / 1369 -17.5 0.20 (1) 6.25 12-11 10.00

11-10

JT.

18

{40-

100083566

WCE OF ONTAR

March 10, 2017

0/0

LOC. -10-12

-17.5

MAX-

-1864

FACTORED CONCENTRATED LOADS (LBS)

LC1

-17.5 0.17 (1)

MAX+

COEL TOLEN'S, DOG, BASED ON THE (M.M. WIND FORCE RESIS WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE

IS AN INTEGRAL PART OF THIS DRAWING AS IT

IN THE DESIGN OF THIS COMPONENT.

CONTAINS SPECIFICATIONS AND CRITERIA USED

10.00

DIR

VERT

TYPF

ERENCE VELOCITY PRESSURE OF { 9.0} PSF A

FERENCE VELOCITY PRESSURE OF COUNTY PRESSURE OF COU

17-4978

**BUILDING DIVISION** 

FACE

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN MAX MIN

MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (18) (INPUT = 0.90) JSI METAL= 0.58 (18) (INPUT = 1.00) Kott Lumber Uxbridge, Stouffville, ON, TW

TW0317-048

TRUSS NAME

QUANTITY PLY **GIRD27** 

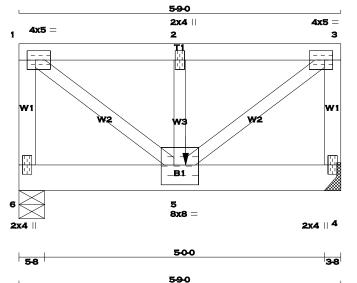
JOBPESC TR-GREENPARK-LECCO RIDGE-BLOCK 327 TRUSS DESC

DRWG NO.

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SCALE = 1:20.6

Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:30 2017 Page 1 ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-I7CFBZTVmRK7RhS9eg29OMSritDpYW7YCwpToSzcJK′



TOTAL WEIGHT = 28 lb

LUMBER				
N. L. G. A. R	ULES			
CHORDS	SIZE		LUMBER	DESCR
6 - 1	2x4	DRY	No.2	SPF
1 - 3	2x4	DRY	No.2	SPF
4 - 3	2x4	DRY	No.2	SPF
6 - 4	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASO	ONED L	UMBER.		

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Υ	X
1	TMVW-t	MT20	4.0	5.0	2.00	1.75
2	TMW+w	MT20	2.0	4.0		
3	TMVW-t	MT20	4.0	5.0	2.00	1.75
4	BMV1+p	MT20	2.0	4.0		
5	BMWWW-t	MT20	8.0	8.0	4.25	4.00
6	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 528.0 lbs FACTORED DOWN AND 239.7
lbs FACTORED UP AT 2-11-12 ON BOTTOM
CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY **BUILDING DESIGNER** 

	FACTOR	RED	MAXIMU	M FACT	ORED	INPUT	REQRD
	GROSS RE	ACTION	GROSS F	REACTIO	BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
6	1302	0	1473	-115	-554	5-8	5-8
4	1679	0	1924	0	-690	HANGER E	BY OTHERS
						MIN. SEAT	SIZE: 3-8

PROVIDE ANCHORAGE AT BEARING JOINT 6 FOR 554 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 4 FOR 690 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES. SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 115 LBS FACTORED HORIZONTAL REACTION AT JOINT 6

UNFACTORED REACTIONS

	1ST LCASE	MAX.	MIN. COMPON	ENT REACTION	ONS		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
6	914	639 / 0	0/0	0/0	429 / -572	275 / 0	0/0
4	1178	824 / 0	0/0	0/0	613 / -721	354 / 0	0/0
HOR 6	IZONTAL REA	ACTIONS 0/0	0/0	0/0	82 / -82	0/0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 6

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.45 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (11)

СН	CHORDS				WEBS				
MA)	X. FACTORED	FACTO	RED				MAX. FACTO	RED	
MEMB.	FORCE	VERT. LO	AD LC	1 MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PL	_F) (	CSI (LC)	<b>UNBRAC</b>	0	(LBS)	CSI (LC)	
FR-TO		FROM	TO		LENGTH	FR-TO			
6- 1	-1165 / 472	0.0	0.0	0.14(1)	7.38	1- 5	-645 / 1720	0.38(1)	
1- 2	-1372 / 552	-77.3	-77.3	0.14 (7)	5.45	5- 2	-256 / 211	0.05 (7)	
2-3	-1372 / 552	-77.3	-77.3	0.14 (7)	5.45	5-3	-645 / 1741	0.38(1)	
4- 3	-1178 / 472	0.0	0.0	0.14 (1)	7.36				
6- 5	-74 / 99	-230.9	-230.9	0.14 (4)	6.25				
5- 4	-17 / 44			0.31 (3)					
FACTO	51.070050 0011051170.1750.1.0.100.4.000								
	FACTORED CONCENTRATED LOADS (LBS)								
IT.		MAY-	MAY			שור	TVDE		

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
5	2-11-12	-455	-528	240	FRONT	VERT	TOTAL

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF ( 9.0) PSP AT (40-00) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM), INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2), BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

**RECEIVED** TOWN OF MILTON MAR 29, 2017 17-4978 **BUILDING DIVISION** 

## **DESIGN CRITERIA**

\*\*\* SPECIAL LOADS ANALYSIS \*\*\*
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

ГOР	CH.	LL	=	23.3	PSF
		DL	=	3.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.0	PSF
$\Gamma \cap T \Delta$	1 10	ΔD	_	333	DSE

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE

GIRDER TYPE: CStdGirder START DISTANCE = 0-0 START SPAN CARRIED = 11-5-8 END DISTANCE = 2-11-12 END SPAN CARRIED = 11-5-8 END WALL WIDTH = 5-8 APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

GIRDER TYPE: CStdGirder START DISTANCE = 2-11-12 START SPAN CARRIED = 22-2-2 END DISTANCE = 5-9-0 END SPAN CARRIED = 22-2-2 END WALL WIDTH = 1-8
APPLIED TO FRONT SIDE OF BOTTOM CHORD. - ADDT'L LOADS BASED ON 55 % OF GSL.

NON STANDARD GIRDER \*\* ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014 - CSA 086-09

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL)= L/999 (0.04")

CSI: TC=0.14 (2-3:7), BC=0.31 (4-5:3), WB=0.38 (1-5:1), SSI=0.52 (4-5:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

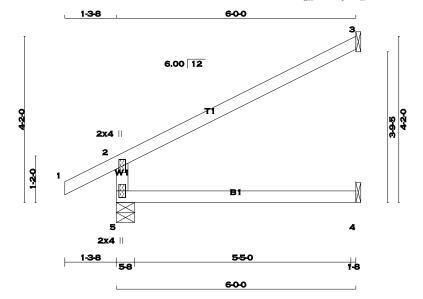
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

JOB HAME    IRUSS NAME   GIRD27   1   1   1   1   1   1   1   1   1	TW0317-048  Ott Lumber Uxbridge, Stouffville, ON, TW  Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:30 2017 Page ID:4BORDhR74Ei?Dog_xdfUkJyKZiI7CFBZTVmRK7RhS9eg29OMSritDpYW7YCwpToSzcJk  NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) MAX MIN MAX MIN MAX MIN MT20 618 354 1667 822 2284 1656  PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg.  JSI GRIP= 0.87 (1) (INPUT = 0.90)
Kott Lumber Uxbridge, Stouffville, ÖN, TW  Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:30 2017 Pa ID:4BORDhR74Ei?Dog_xdfUkJyKZiI7CFBZTVmRK7RhS9eg29OMSritDpYW7YCwpToSzc  NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MAX MIN MAX MIN MT20 618 354 1667 822 2284 1656  PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg.  JSI GRIP= 0.87 (1) (INPUT = 0.90)	D. 480R0-bit / 26T   D. 28T   D. 2
NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MAX MIN MT20 618 354 1667 822 2284 1656 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg.  JSI GRIP= 0.87 (1) (INPUT = 0.90)	MALVALUE (1997) BFAS SPCTION (1998) TUBER
PLATE GRIP(DRY) SHEAR SECTION (PS) (PLI) (PLI) (PS) (PLI) (PLI) (PS) (PLI) (PLI) (MAX MIN MAX MIN MAX MIN MAX MIN MT20 618 354 1667 822 2284 1656 (PLATE PLACEMENT TOL. = 0.250 inches (PLATE ROTATION TOL. = 5.0 Deg.  JSI GRIP= 0.87 (1) (INPUT = 0.90)	PLATE SERVICION (ASS, TOM, ASS, ASS, ASS, ASS, ASS, ASS, ASS, AS

POBPES GREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY PLY DRWG NO. TRUSS DESC. TW0317-048 J01 22

Kott Lumber Uxbridge, Stouffville, ON, TW

Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:30 2017 Page 1 ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-I7CFBZTVmRK7RhS9eg29OMSmWtFgYb\_YCwpToSzcJK'



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR SIZE 2x4 DRY No.2 No.2 SPF SPF 2x4 DRY No 2

DRY: SEASONED LUMBER

PLATES (table is in inches)

TYPE TMV+p PLATES MT20 W 2.0 LEN Y 4.0 BMV1+p 2.0 4.0

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER **BEARINGS** 

	FACTORED		MAXIMU	M FACT	INPUT	REQRD	
	GROSS R	EACTION	GROSS	REACTIO	N	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
5	457	0	465	273	-210	5-8	5-8
3	174	0	185	0	-179	1-8	1-8
4	43	0	49	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) 3, 4

PROVIDE ANCHORAGE AT BEARING JOINT 5 FOR 210 LBS FACTORED PROVIDE ANCHORAGE AT BEARING JOINT 3 FOR 179 LBS FACTORED

PROVIDE FOR 273 LBS FACTORED HORIZONTAL REACTION AT JOINT 5

UNFACTORED REACTIONS

	1ST LCASE	MAX./I	MIN. COMPON	<u>NENT REACTIO</u>	DNS		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
5	318	238 / 0	0/0	0/0	22 / -202	80 / 0	0/0
3	118	105/0	0/0	0/0	28 / -137	14/0	0/0
4	35	0/0	0/0	0/0	1 / -4	35 / 0	0/0
HOR 5	IZONTAL REA	ACTIONS 0/0	0/0	0/0	195 / -62	0/0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 5

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (11)

	R D S FACTORED	FACTORED	)		WE	BS MAX. FACTO	RED	
MEMB.	FORCE	VERT. LOAD	LC1 MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PLF)	CSI (LC)	<b>UNBRAC</b>	0	(LBS)	CSI (LC)	
FR-TO		FROM TO		LENGTH	FR-TO	, ,		
5-2	-404 / 260	0.0	0.0 0.21 (7)	7.81				
1-2	0 / 23	-77.3 -77	7.3 0.10 (1)	10.00				
2-3	-87 / 3	-77.3 -77	7.3 0.47 (1)	6.25				
5- 4	0/0	-17.5 -17	7.5 0.13 (11	) 10.00				

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

# TOTAL WEIGHT = 22 X 17 = 376 lb **DESIGN CRITERIA**

SPECIFIED LOADS: CH.

LL = DL = LL = DL = AD = 3.0 PSF 7.0 PSF TOTAL LOAD 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

PSF

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SCALE = 1:28.9

TW0317-048

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014

- CSA 086-09 TPIC 2011

DESIGN ASSUMPTIONS -OVERHANG NOT TO BE ALTERED OR CUT

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20") CALCULATED VERT. DEFL.(LL)= L/ 999 (0.01") ALLOWABLE DEFL.(TL)= L/360 (0.20") CALCULATED VERT. DEFL.(TL)= L/999 (0.03")

CSI: TC=0.47 (2-3:1), BC=0.13 (4-5:11), WB=0.00 (n/a:0), SSI=0.20 (2-3:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.33 (2) (INPUT = 0.90 ) JSI METAL= 0.18 (5) (INPUT = 1.00)



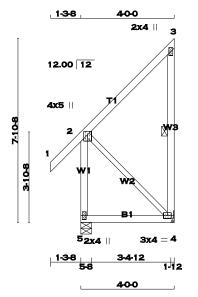


READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

JOB NAME TRUSS NAME QUANTITY PLY DRWG NO. JOBPESC TR-GREENPARK-LECCO RIDGE-BLOCK 327 TRUSS DESC TW0317-048 J02

Kott Lumber Uxbridge, Stouffville, ON, TW

Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:31 2017 Page ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-DJmePvU7XIS\_2r1LCNZOxa?zIHceH1ahRaY0KuzcJK



TOTAL WEIGHT = 4 X 30 = 118 lb

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SCALE = 1:49.3

TW0317-048

LUMBER				
N. L. G. A. F	RULES			
CHORDS	SIZE		LUMBER	DESCR
1 - 3	2x4	DRY	No.2	SPF
4 - 3	2x4	DRY	No.2	SPF
5 - 2	2x4	DRY	No.2	SPF
5 - 4	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEAS	ONED L	UMBER.		

PLATES (table is in inches)

PLATES W LEN Y 1.75 2.00 TMVW+p 4.0 5.0 3 TMV+p MT20 2.0 4.0 BMVW1-t BMV1+p MT20 2.0 4.0

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS

	FACTOR	RED	MAXIMUI	MAXIMUM FACTORED			REQRD			
	GROSS RE	ACTION	GROSS REACTION			BRG	BRG			
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX			
4	190	0	338	0	-501	HANGER E	BY OTHERS			
						MIN. SEAT	SIZE: 1-12			
5	298	0	415	480	-251	5-8	5-8			

PROVIDE ANCHORAGE AT BEARING JOINT 4 FOR 501 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 5 FOR 251 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES. SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 480 LBS FACTORED HORIZONTAL REACTION AT JOINT 5

UNFACTORED REACTIONS

	1ST LCASE	MAX./	MIN. COMPOR	NENT REACTION	ONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL			
4	133	93 / 0	0/0	0/0	272 / -384	40 / 0	0/0			
5	206	158 / 0	0/0	0/0	294 / -210	48 / 0	0/0			
HORIZONTAL REACTIONS										
5		0/0	0/0	0/0	343 / -262	0/0	0 /0			

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 5

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 3-4. DBS = 20-0-0. CBF = 21 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING TOTAL LOAD CASES: (11)

CHO	DRDS		WEBS					
MAX	FACTORED	FACTO	RED				MAX. FACTO	DRED
MEMB.	FORCE	VERT. LO	AD LC	1 MAX	MAX.	MEMB.	FORCE	MAX
	(LBS)	(PL	.F) (	CSI (LC)	UNBRAC		(LBS)	CSI (LC)
FR-TO		FROM	TO		LENGTH	FR-TO		
1- 2	0/38	-77.3	-77.3	0.11 (7)	10.00	2-4	-250 / 456	0.11 (5)
2-3	-216 / 201	-77.3	-77.3	0.31 (7)	6.25			
4- 3	-197 / 226	0.0	0.0	0.34 (7)	6.25			
5- 2	-380 / 276	0.0	0.0	0.09(4)	7.81			
5- 4	-411 / 335	-17.5	-17.5	0.08 (11	6.25			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {440-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM, INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

RECEIVED TOWN OF MILTON MAR 29, 2017 17-4978 **BUILDING DIVISION**  **DESIGN CRITERIA** 

SPECIFIED LOADS: LL = DL = LL = DL = AD = CH. PSF 3.0 PSF 7.0 PSF TOTAL LOAD 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012, BCBC 2012, ABC 2014 - CSA 086-09

- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.19") CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.34 (3-4:7) , BC=0.08 (4-5:11) , WB=0.11 (2-4:5) , SSI=0.13 (2-3:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.63 (4) (INPUT = 0.90 ) JSI METAL= 0.15 (4) (INPUT = 1.00 )



JOB NAME TRUSS NAME

TW0317-048

J03 Kott Lumber Uxbridge, Stouffville, ON, TW

QUANTITY PLY

TRUSS DESC.

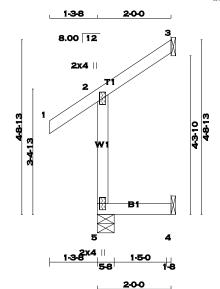
JOB PR-GREENPARK-LECCO RIDGE-BLOCK 327

DRWG NO.

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SCALE = 1:31.2

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TOTAL WEIGHT = 4 X 10 = 42 lb

LUMBER				
N. L. G. A. I	RULES			
CHORDS	SIZE		LUMBER	DESCR
5 - 2	2x4	DRY	No.2	SPF
1 - 3	2x4	DRY	No.2	SPF
5 - 4	2x4	DRY	No.2	SPF
•				

DRY: SEASONED LUMBER.

# PLATES (table is in inches)

PLATES MT20 W 2.0 TYPE TMV+p LEN Y 4.0 BMV1+p 2.0 4.0

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER **BEARINGS** 

	FACTOR	ED	MAXIMU	M FACTO	INPUT	REQRD	
	GROSS RE	ACTION	GROSS F	REACTIO	BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
5	220	0	220	259	0	5-8	5-8
3	59	0	71	0	-70	1-8	1-8
4	17	0	49	0	-127	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) 3, 4

PROVIDE ANCHORAGE AT BEARING JOINT 3 FOR 150 LBS FACTORED PROVIDE ANCHORAGE AT BEARING JOINT 4 FOR 150 LBS FACTORED

PROVIDE FOR 259 LBS FACTORED HORIZONTAL REACTION AT JOINT 5

## UNFACTORED REACTIONS

	1ST LCASE	MAX./	<u>MIN. COMPON</u>	<u>NENT REACTIO</u>	DNS		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
5	152	121/0	0/0	0/0	20 / -44	30 / 0	0/0
3	40	36 / 0	0/0	0/0	29 / -53	5/0	0/0
4	13	0/0	0/0	0/0	81 / -99	13 / 0	0/0
HOR 5	IZONTAL REA	ACTIONS 0/0	0/0	0/0	185 / -170	0/0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 5

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (12)

CHO	RDS			WEBS						
MAX.	FACTORED	FACTORED			MAX. FACTORED					
MEMB.	FORCE	VERT. LO	AD LC	1 MAX	MAX.	MEMB.	FORCE	MAX		
	(LBS)	(PL	.F) (	CSI (LC)	<b>UNBRAC</b>		(LBS)	CSI (LC)		
FR-TO		FROM	TO		LENGTH	FR-TO				
5- 2	-216 / 119	0.0	0.0	0.41 (7)	7.81					
1- 2	0/29	-77.3	-77.3	0.10(1)	10.00					
2- 3	-42 / 5	-77.3	-77.3	0.05 (7)	6.25					
5- 4	0/0	-17.5	-17.5	0.33 (7)	10.00					

# CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

# **DESIGN CRITERIA**

SPECIFIED LOADS:

LL = DL = LL = DL = AD = CH. PSF 3.0 PSF 7.0 PSF TOTAL LOAD 33.3 PSF

## SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014 - CSA 086-09

- TPIC 2011

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19") CALCULATED VERT. DEFL.(LL)= L/ 999 (0.01") ALLOWABLE DEFL.(TL)= L/360 (0.19") CALCULATED VERT. DEFL.(TL)= L/999 (0.01")

CSI: TC=0.41 (2-5:7), BC=0.33 (4-5:7), WB=0.00 (n/a:0), SSI=0.18 (2-5:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.26 (5) (INPUT = 0.90) JSI METAL= 0.17 (5) (INPUT = 1.00)



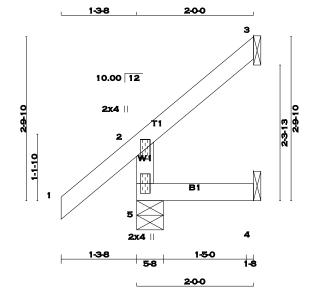


READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

POBPES GREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY PLY DRWG NO. TRUSS DESC. TW0317-048 J04 6

Kott Lumber Uxbridge, Stouffville, ON, TW

Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:31 2017 Page ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-DJmePvU7XIS\_2r1LCNZOxa?0UHd5H2EhRaY0KuzcJK



TOTAL WEIGHT = 6 X 8 = 50 lb

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SCALE = 1:19.7

TW0317-048

LUMBER				
N. L. G. A. I	RULES			
CHORDS	SIZE		LUMBER	DESC
5 - 2	2x4	DRY	No.2	SPF
1 - 3	2x4	DRY	No.2	SPF
5 - 4	2x4	DRY	No.2	SPF
				٠.

DRY: SEASONED LUMBER.

# PLATES (table is in inches)

PLATES MT20 W 2.0 TYPE TMV+p LEN Y 4.0 BMV1+p 2.0 4.0

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

## DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER **BEARINGS**

	FACTO	RED	MAXIMU	M FACT	INPUT	REQRD	
	GROSS R	EACTION	GROSS	REACTIO	BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
5	223	0	232	205	-73	5-8	5-8
3	58	0	72	0	-82	1-8	1-8
4	16	0	19	0	-10	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) 3, 4

PROVIDE ANCHORAGE AT BEARING JOINT 5 FOR 150 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 3 FOR 150 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 4 FOR 150 LBS FACTORED UPLIFT

PROVIDE FOR 205 LBS FACTORED HORIZONTAL REACTION AT JOINT 5

# UNFACTORED REACTIONS

	1ST LCASE	ST LCASE MAX./MIN. COMPONENT REACTIONS									
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL				
5	154	122 / 0	0/0	0/0	24 / -72	31 / 0	0/0				
3	40	35 / 0	0/0	0/0	33 / -62	5/0	0/0				
4	12	0/0	0/0	0/0	8 / -15	12/0	0/0				
HOR 5	RIZONTAL REA	ACTIONS 0/0	0/0	0/0	147 / -87	0/0	0 /0				

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 5

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (12)

CHC	RDS			WEBS					
MAX.	FACTORED	FACTO	RED			N	MAX. FACTO	RED	
MEMB.	FORCE	VERT. LO	AD LC	1 MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PL	.F) (	CSI (LC)	<b>UNBRA</b>	С	(LBS)	CSI (LC)	
FR-TO		FROM	TO		LENGTI	H FR-TO			
5-2	-216 / 108	0.0	0.0	0.14 (7)	7.81				
1- 2	0 / 34	-77.3	-77.3	0.11(1)	10.00				
2-3	-56 / 7	-77.3	-77.3	0.07 (7)	6.25				
5- 4	0/0	-17.5	-17.5	0.05 (7)	10.00				

# CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT WIND LOAD TAPPLED IS DERIVED FROM REPERSING VELOCITY FRESSURE OF (\$3.0) FOR AL (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

**RECEIVED** TOWN OF MILTON MAR 29, 2017 17-4978 **BUILDING DIVISION** 

## **DESIGN CRITERIA**

SPEC	IFIED	LOAD	S:		
TOP	CH.	LL	=	23.3	PSF
		DL	=	3.0	PSF
BOT	CH	- 11	=	0.0	PSF

DL = 7.0 AD = 33.3 PSF TOTAL LOAD PSF

## 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014

- CSA 086-09 - TPIC 2011

# DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL)= L/999 (0.00")

CSI: TC=0.14 (2-5:7), BC=0.05 (4-5:7), WB=0.00 (n/a:0), SSI=0.14 (2-5:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

# NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

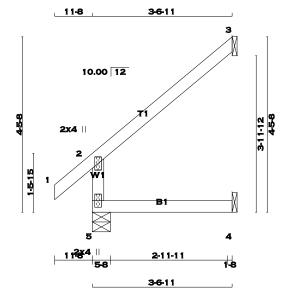
JSI GRIP= 0.22 (2) (INPUT = 0.90) JSI METAL= 0.13 (5) (INPUT = 1.00)



JOB PR-GREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY PLY DRWG NO. TRUSS DESC. 8 TW0317-048 J05

Kott Lumber Uxbridge, Stouffville, ON, TW

Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:31 2017 Page ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-DJmePvU7XIS\_2r1LCNZOxa?\_GHcGH2EhRaY0KuzcJK



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR SIZE 2x4 DRY No.2 No.2 SPF SPF DRY No 2

DRY: SEASONED LUMBER

PLATES (table is in inches)

TYPE TMV+p PLATES MT20 W 2.0 LEN Y 4.0 BMV1+p 2.0 4.0

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS

	FACTO	RED	MAXIMU	M FACT	INPUT	REQRD	
	GROSS R	EACTION	GROSS	REACTIO	BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
5	288	0	302	293	-78	5-8	5-8
3	103	0	127	0	-147	1-8	1-8
4	27	0	31	0	-3	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) 3, 4

PROVIDE ANCHORAGE AT BEARING JOINT 5 FOR 150 LBS FACTORED PROVIDE ANCHORAGE AT BEARING JOINT 3 FOR 150 LBS FACTORED PROVIDE ANCHORAGE AT BEARING JOINT 4 FOR 150 LBS FACTORED UPLIFT

PROVIDE FOR 293 LBS FACTORED HORIZONTAL REACTION AT JOINT 5

UNFACTORED REACTIONS

	1ST LCASE	MAX.	MIN. COMPON	NENT REACTION	ONS		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
5	200	152 / 0	0/0	0/0	34 / -86	48 / 0	0/0
3	70	62 / 0	0/0	0/0	59 / -110	8/0	0/0
4	22	0/0	0/0	0/0	9 / -16	22 / 0	0/0
HORIZONTAL REACTIONS 5 0/0 0/0 0/0 209/-133 0/0							

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 5

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (11)

CHO	DRDS			WEBS				
MAX	FACTORED	FACTORED		MAX. FACTORED				
MEMB.	FORCE	VERT. LOAD I	LC1 MAX	MAX. ME	MB.	FORCE	MAX	
	(LBS)	(PLF)	CSI (LC)	UNBRAC		(LBS)	CSI (LC)	
FR-TO		FROM TO		LENGTH FR-	-TO			
5- 2	-271 / 126	0.0	0.0 0.28 (7)	7.81				
1- 2	0 / 26	-77.3 -77	'.3 0.06 (1)	10.00				
2-3	-100 / 13	-77.3 -77	'.3 0.21 (7)	6.25				
5- 4	0/0	-17.5 -17	'.5 0.11 (7)	10.00				

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT { 40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

## **DESIGN CRITERIA**

SPECIFIED LOADS: CH. PSF PSF

LL = DL = LL = DL = AD = 3.0 7.0 PSF TOTAL LOAD 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

TOTAL WEIGHT = 8 X 12 = 98 lb

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SCALE = 1:29.3

TW0317-048

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014

- CSA 086-09 - TPIC 2011

DESIGN ASSUMPTIONS -OVERHANG NOT TO BE ALTERED OR CUT

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL)= L/999 (0.01")

CSI: TC=0.28 (2-5:7), BC=0.11 (4-5:7), WB=0.00 (n/a:0), SSI=0.20 (2-5:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.30 (2) (INPUT = 0.90 ) JSI METAL= 0.19 (5) (INPUT = 1.00)





READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

JOB PR-GREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY PLY DRWG NO. Page 47 of 159 TRUSS DESC. 2 TW0317-048 TW0317-048 J06 Kott Lumber Uxbridge, Stouffville, ON, TW Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:32 2017 Page 1 ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-hVK0cFVII2arg\_cXm44dTnXBDhx80VUqgEIZrLzcJJz 207 1.3-8 SCALE = 1:18.9 3 8.00 12 2x4 || K) 4 8

LUMBER										
N. L. G. A. RULES										
CHORDS	SIZE		LUMBER	DESCR.						
5 - 2	2x4	DRY	No.2	SPF						
1 - 3	2x4	DRY	No.2	SPF						
5 - 4	2x4	DRY	No.2	SPF						
DRY: SEASONED LUMBER.										

PL/	ATES (tab	le is in inche	<u>s)</u>		
JT	TYPE	PLATES	W	LEN Y	Х
2	TMV+p	MT20	2.0	4.0	
5	BMV1+p	MT20	2.0	4.0	

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS

600

1-8

3-10-1

BLAKINGS											
	FACTORED		MAXIMUN	M FACTO	INPUT	REQRD					
	<b>GROSS RE</b>	ACTION	GROSS F	REACTIO	BRG	BRG					
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX				
5	265	0	279	183	-72	3-0	3-0				
3	60	0	71	0	-71	1-8	1-8				
4	44	0	49	0	0	1-8	1-8				

1-7-15

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) 3, 4

PROVIDE ANCHORAGE AT BEARING JOINT 5 FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 3 FOR 150 LBS FACTORED UPLIFT

PROVIDE FOR 183 LBS FACTORED HORIZONTAL REACTION AT JOINT 5

UNF	UNFACTORED REACTIONS											
	1ST LCASE	MAX./N	MAX./MIN. COMPONENT REACTIONS									
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL					
5	188	123 / 0	0/0	0/0	35 / -93	65 / 0	0/0					
3	40	36 / 0	0/0	0/0	29 / -54	5/0	0/0					
4	35	0/0	0/0	0/0	2 / -4	35 / 0	0/0					
HORIZONTAL REACTIONS												
5		0/0	0/0	0/0	130 / -88	0/0	0 /0					

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 5, 3

2x4

1.3-8

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (11)

	R D S FACTORED	FACTORE	)		WEE	S MAX. FACTO	RED	
MEMB.	FORCE	VERT. LOAD	LC1 MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PLF)	CSI (LC)	UNBRA	0	(LBS)	CSI (LC)	
FR-TO		FROM TO		LENGTH	FR-TO		, ,	
5-2	-219 / 121	0.0	0.0 0.14 (7)	7.81				
1-2	0 / 29	-77.3 -7	7.3 0.10 (1)	10.00				
2-3	-42 / 5	-77.3 -7	7.3 0.04 (7)	6.25				
5- 4	0/0	-17.5 -1	7.5 0.13 (1	1) 10.00				

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

## **DESIGN CRITERIA**

SPECIFIED LOADS: LL = DL = LL = DL = AD = CH. 3.0 PSF

7.0 PSF TOTAL LOAD 33.3 PSF

## SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

TOTAL WEIGHT = 2 X 13 = 25 lb

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014

- CSA 086-09 - TPIC 2011

DESIGN ASSUMPTIONS -OVERHANG NOT TO BE ALTERED OR CUT

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20") CALCULATED VERT. DEFL.(LL)= L/ 999 (0.01") ALLOWABLE DEFL.(TL)= L/360 (0.20") CALCULATED VERT. DEFL.(TL)= L/999 (0.04")

CSI: TC=0.14 (2-5:7), BC=0.13 (4-5:11), WB=0.00 (n/a:0), SSI=0.12 (2-5:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (5) (INPUT = 0.90 ) JSI METAL= 0.12 (5) (INPUT = 1.00)



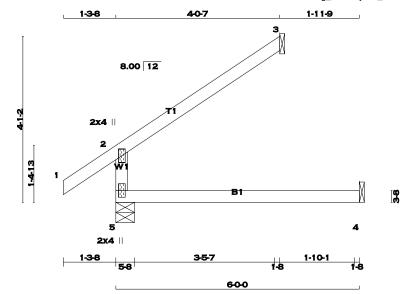


READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

JOB PESC TR-GREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY PLY DRWG NO Page 48 of 159 TRUSS DESC. 2 TW0317-048 TW0317-048 J07

Kott Lumber Uxbridge, Stouffville, ON, TW

Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:32 2017 Page 1 ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-hVK0cFVII2arg\_cXm44dTnXAbhx80VUqgEIZrLzcJJz



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR SIZE 2x4 DRY No.2 No.2 SPF SPF 2x4 DRY No 2

DRY: SEASONED LUMBER

PLATES (table is in inches)

TYPE TMV+p PLATES MT20 W 2.0 LEN Y 4.0 BMV1+p 2.0 4.0

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS

DLA	BLAKINGS											
	FACTORED		MAXIMU	M FACT	INPUT	REQRD						
	GROSS R	EACTION	GROSS	REACTIO	BRG	BRG						
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX					
5	362	0	385	273	-125	5-8	5-8					
3	117	0	140	0	-141	1-8	1-8					
4	44	0	49	0	0	1-8	1-8					

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) 3, 4

PROVIDE ANCHORAGE AT BEARING JOINT 5 FOR 150 LBS FACTORED
PROVIDE ANCHORAGE AT BEARING JOINT 3 FOR 150 LBS FACTORED

PROVIDE FOR 273 LBS FACTORED HORIZONTAL REACTION AT JOINT 5

UNFACTORED REACTIONS

	1ST LCASE	MAX./I	<u>MIN. COMPON</u>	<u>NENT REACTIO</u>	ONS			
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
5	254	181 / 0	0/0	0/0	58 / -135	72 / 0	0/0	
3	80	71 / 0	0/0	0/0	57 / -106	9/0	0/0	
4	35	0/0	0/0	0/0	3 / -6	35 / 0	0/0	
HORIZONTAL REACTIONS 5 0/0 0/0 0/0 195/-123 0/0 0/0								

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 5. 3

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (11)

CHC	RDS				WEBS				
MAX.	FACTORED	FACTOR	RED			1	MAX. FACTO	RED	
MEMB.	FORCE	VERT. LO	AD LC1	1 MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PL	F) (	CSI (LC)	UNBRAC		(LBS)	CSI (LC)	
FR-TO		FROM	TO		LENGTH	FR-TO			
5-2	-326 / 177	0.0	0.0	0.25(7)	7.81				
1-2	0 / 29	-77.3	-77.3	0.10(1)	10.00				
2-3	-84 / 11	-77.3	-77.3	0.22 (7)	6.25				
5- 4	0/0	-17.5	-17.5	0.13 (11)	10.00				

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

## **DESIGN CRITERIA**

SPECIFIED LOADS: CH.

LL = DL = LL = DL = AD = 3.0 PSF 7.0 PSF TOTAL LOAD 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

TOTAL WEIGHT = 2 X 15 = 31 lb

PSF

SCALE = 1:28.4

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014

- CSA 086-09 TPIC 2011

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20") CALCULATED VERT. DEFL.(LL)= L/ 999 (0.01") ALLOWABLE DEFL.(TL)= L/360 (0.20") CALCULATED VERT. DEFL.(TL)= L/999 (0.04")

CSI: TC=0.25 (2-5:7), BC=0.13 (4-5:11), WB=0.00 (n/a:0), SSI=0.19 (2-5:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.29 (2) (INPUT = 0.90 ) JSI METAL= 0.18 (5) (INPUT = 1.00)



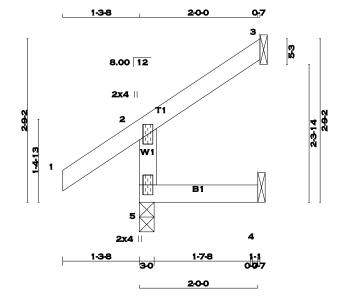


READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

TRUSS NAME POBPES GREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME QUANTITY PLY DRWG NO. 3 TW0317-048 J08

Kott Lumber Uxbridge, Stouffville, ON, TW

TW0317-048 Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:32 2017 Page ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-hVK0cFVII2arg\_cXm44dTnXB1hy40VUqgEIZrLzcJJz



TOTAL WEIGHT = 3 X 8 = 24 lb

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SCALE = 1:19.4

LUMBER				
N. L. G. A. F	RULES			
CHORDS	SIZE		LUMBER	DESCR
5 - 2	2x4	DRY	No.2	SPF
1 - 3	2x4	DRY	No.2	SPF
5 - 4	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

TYPE TMV+p PLATES MT20 W 2.0 LEN Y 4.0 BMV1+p 2.0 4.0

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS

	FACTORED		MAXIMU	M FACT	INPUT	REQRD	
	GROSS R	EACTION	GROSS	REACTIO	BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
5	223	0	233	183	-79	3-0	3-0
3	60	0	71	0	-71	1-8	1-8
4	16	0	21	0	-17	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) 3, 4

PROVIDE ANCHORAGE AT BEARING JOINT 5 FOR 150 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 3 FOR 150 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 4 FOR 150 LBS FACTORED UPLIFT

PROVIDE FOR 183 LBS FACTORED HORIZONTAL REACTION AT JOINT 5

UNFACTORED REACTIONS

	1ST LCASE	MAX./	MIN. COMPON	NENT REACTION	ONS		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
5	154	123 / 0	0/0	0/0	25 / -76	31 / 0	0/0
3	40	36 / 0	0/0	0/0	29 / -54	5/0	0/0
4	13	0/0	0/0	0/0	12 / -20	13 / 0	0/0
HOF 5	RIZONTAL REA	ACTIONS 0/0	0/0	0/0	130 / -88	0/0	0 /0

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

C H O R D S MAX. FACTORED FACTORED					W E B S MAX. FACTORED				
MEMB.	FORCE	VERT. LO	AD LC	1 MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PLI	F) (	CSI (LC)	UNBRA	С	(LBS)	CSI (LC)	
FR-TO		FROM	TO		LENGTH	HFR-TO		, ,	
5-2	-219 / 121	0.0	0.0	0.15 (7)	7.81				
1- 2	0 / 29	-77.3	-77.3	0.10(1)	10.00				
2-3	-42 / 5	-77.3	-77.3	0.06 (7)	6.25				
5- 4	0/0	-17.5	-175	0.07 (7)	10.00				

# CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT WIND LOAD TAPPLED IS DERIVED FROM REPERSING VELOCITY FRESSURE OF (\$3.0) FOR AL (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.

# BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 5 BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY LOADING TOTAL LOAD CASES: (12)



March 10, 2017



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

**RECEIVED** TOWN OF MILTON MAR 29, 2017 17-4978 **BUILDING DIVISION** 

## **DESIGN CRITERIA**

SPECIFIED LOADS: TOP CH.

LL = DL = LL = DL = AD = 3.0 PSF 7.0 PSF TOTAL LOAD 33.3

## SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014

- CSA 086-09 - TPIC 2011

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL)= L/999 (0.00")

CSI: TC=0.15 (2-5:7), BC=0.07 (4-5:7), WB=0.00 (n/a:0), SSI=0.12 (2-5:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (5) (INPUT = 0.90) JSI METAL= 0.12 (5) (INPUT = 1.00 ) JOB NAME TRUSS NAME

Kott Lumber Uxbridge, Stouffville, ON, TW

TW0317-048

QUANTITY PLY 2 J09

1-3-8

POBPES GREENPARK-LECCO RIDGE-BLOCK 327 TRUSS DESC

DRWG NO.

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SCALE = 1:27.4

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က် 8.00 12 2x4 ||

200

LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER SIZE DESCR 2 2x4 DRY No.2 No.2 SPF SPF DRY No 2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

TYPE TMV+p PLATES MT20 W 2.0 LEN Y 4.0 BMV1+p 2.0 4.0

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS

1-10-15

DEM	KINGS						
	FACTORED		MAXIMU	M FACT	INPUT	REQRD	
	GROSS R	EACTION	GROSS	REACTIO	BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
5	320	0	338	273	-120	5-8	5-8
3	117	0	140	0	-141	1-8	1-8
4	16	0	23	0	-32	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) 3, 4

200

PROVIDE ANCHORAGE AT BEARING JOINT 5 FOR 150 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 3 FOR 150 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 4 FOR 150 LBS FACTORED UPLIFT

PROVIDE FOR 273 LBS FACTORED HORIZONTAL REACTION AT JOINT 5

UNFACTORED REACTIONS

	1ST LCASE	MAX.	MIN. COMPON	NENT REACTION	DNS		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
5	220	181 / 0	0/0	0/0	43 / -110	39 / 0	0/0
3	80	71 / 0	0/0	0/0	57 / -106	9/0	0/0
4	13	0/0	0/0	0/0	18 / -31	13 / 0	0/0
HOF 5	RIZONTAL RE.	ACTIONS 0/0	0/0	0/0	195 / -123	0/0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 5

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (11)

	R D S FACTORED	FACTORED		W E B S MAX. FACTORED				
MEMB.	FORCE	VERT. LOAD L	C1 MAX	MAX. I	MEMB.	FORCE	MAX	
	(LBS)	(PLF)	CSI (LC)	UNBRAC		(LBS)	CSI (LC)	
FR-TO		FROM TO		LENGTH F	R-TO			
5-2	-326 / 177	0.0 0	.0 0.24 (7)	7.81				
1- 2	0 / 29	-77.3 -77	.3 0.10 (1)	10.00				
2-3	-84 / 11	-77.3 -77	.3 0.22 (7)	6.25				
5- 4	0/0	-17.5 -17	.5 0.11 (7)	10.00				

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT { 40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

## **DESIGN CRITERIA**

SPECIFIED LOADS: TOP CH. PSF

LL = DL = LL = DL = AD = 3.0 7.0 PSF TOTAL LOAD 33.3

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

TOTAL WEIGHT = 2 X 11 = 22 lb

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014

- CSA 086-09 - TPIC 2011

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL)= L/999 (0.00")

CSI: TC=0.24 (2-5:7), BC=0.11 (4-5:7), WB=0.00 (n/a:0), SSI=0.19 (2-5:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.29 (2) (INPUT = 0.90) JSI METAL= 0.18 (5) (INPUT = 1.00)





READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

TRUSS NAME JOB NAME

Kott Lumber Uxbridge, Stouffville, ON, TW

J10

TW0317-048

QUANTITY PLY 3

JOB PESC TR-GREENPARK-LECCO RIDGE-BLOCK 327

DRWG NO.

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1-3-8 6-0-0 8.00 12 4-11-10 2x4 || B1 2x4 || 5.7.8

SCALE = 1:36.0

LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR SIZE 2x4 DRY No.2 No.2 SPF SPF DRY No 2

DRY: SEASONED LUMBER

PLATES (table is in inches) TYPE TMV+p PLATES MT20 W 2.0 LEN Y 4.0 BMV1+p 2.0 4.0

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS

600

FACTORED		MAXIMU	M FACT	INPUT	REQRD	
GROSS R	EACTION	GROSS I	REACTIO	BRG	BRG	
VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
457	0	489	362	-176	3-0	3-0
174	0	208	0	-209	1-8	1-8
44	0	49	0	0	1-8	1-8
	GROSS RI VERT 457 174	GROSS REACTION VERT HORZ 457 0 174 0	GROSS REACTION GROSS I VERT HORZ DOWN 457 0 489 174 0 208	GROSS REACTION GROSS REACTION VERT HORZ DOWN HORZ 457 0 489 362 174 0 208 0	GROSS REACTION GROSS REACTION VERT HORZ DOWN HORZ UPLIFT 457 0 489 362 -176 174 0 208 0 -209	GROSS REACTION VERT         GROSS REACTION DOWN         BRG HORZ         BRG UPLIFT           457         0         489         362         -176         3-0           174         0         208         0         -209         1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) 3, 4

PROVIDE ANCHORAGE AT BEARING JOINT 5 FOR 176 LBS FACTORED PROVIDE ANCHORAGE AT REARING JOINT 3 FOR 209 LBS FACTORED

PROVIDE FOR 362 LBS FACTORED HORIZONTAL REACTION AT JOINT 5

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS								
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
5	318	239 / 0	0/0	0/0	80 / -177	80 / 0	0/0	
3	118	105 / 0	0/0	0/0	85 / -158	14 / 0	0/0	
4	35	0/0	0/0	0/0	5 / -8	35 / 0	0/0	
HOR 5	RIZONTAL RE	ACTIONS 0/0	0/0	0/0	259 / -158	0/0	0 /0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 5

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (11)

CHC	RDS				WEBS				
MAX.	FACTORED	FACTOR	RED				MAX. FACTO	RED	
MEMB.	FORCE	VERT. LO	AD LC1	1 MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PL	F) (	CSI (LC)	UNBRAC		(LBS)	CSI (LC)	
FR-TO		FROM	TO		LENGTH	FR-TO			
5-2	-430 / 231	0.0	0.0	0.35 (7)	7.81				
1-2	0 / 29	-77.3	-77.3	0.10(1)	10.00				
2-3	-125 / 16	-77.3	-77.3	0.38 (7)	6.25				
5- 4	0/0	-17.5	-17.5	0.13 (11)	10.00				

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM},INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

## **DESIGN CRITERIA**

SPECIFIED LOADS:

LL = DL = LL = DL = AD = CH. PSF 3.0 PSF 7.0 PSF TOTAL LOAD 33.3 PSF

## SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

TOTAL WEIGHT = 3 X 18 = 54

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014

- CSA 086-09 - TPIC 2011

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20") CALCULATED VERT. DEFL.(LL)= L/ 999 (0.02") ALLOWABLE DEFL.(TL)= L/360 (0.20") CALCULATED VERT. DEFL.(TL)= L/999 (0.04")

CSI: TC=0.38 (2-3:7), BC=0.13 (4-5:11), WB=0.00 (n/a:0), SSI=0.25 (2-5:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.40 (2) (INPUT = 0.90) JSI METAL= 0.24 (5) (INPUT = 1.00)





READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

TRUSS NAME POBPES GREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME QUANTITY PLY DRWG NO. TW0317-048 J11 8

Kott Lumber Uxbridge, Stouffville, ON, TW

LUMBER N. L. G. A. RULES CHORDS SIZE

BMV1+p

SIZE

DRY

DRY

PLATES MT20

TEE-LOK TL20 PLATES IS ALLOWED.

W 2.0

2.0 4.0

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH

2x4

DRY: SEASONED LUMBER.

PLATES (table is in inches) TYPE TMV+p

LUMBER

No.2 No.2

LEN Y 4.0

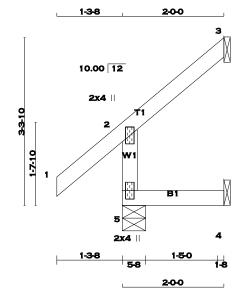
DESCR

SPF SPF

Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:33 2017 Page ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-9itOqaWN3Mjil8BkJobs0?4Lr5lilyk\_uu17NnzcJJy

Page 52 of 159 TW0317-048

SCALE = 1:22.7



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS FACTORED

MAXIMUM FACTORED INPLIT REQRD GROSS REACTION GROSS REACTION LIPLIFT IN-SX JT. VFRT HOR7 DOWN HOR7 IN-SX 5 222 227 225 -49 5-8 5-8 59 0 72 0 -82 1-8 1-8 0 1-8 -33

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) 3, 4

PROVIDE ANCHORAGE AT BEARING JOINT 5 FOR 150 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 3 FOR 150 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 4 FOR 150 LBS FACTORED UPLIFT

PROVIDE FOR 225 LBS FACTORED HORIZONTAL REACTION AT JOINT 5

UNFACTORED REACTIONS

	1ST LCASE	MAX./	MIN. COMPON	NENT REACTION	ONS		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
5	153	122 / 0	0/0	0/0	13 / -55	31 / 0	0/0
3	40	35 / 0	0/0	0/0	33 / -62	5/0	0/0
4	13	0/0	0/0	0/0	19 / -32	13 / 0	0/0
HOF 5	RIZONTAL REA	ACTIONS 0/0	0/0	0/0	161 / -108	0/0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 5, 3

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (12)

C H O R D S MAX. FACTORED FACTORED					W E B S MAX. FACTORED				
MEMB.	FORCE	VERT. LOA	D LC	1 MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PLF	) (	CSI (LC)	UNBRA	С	(LBS)	CSI (LC)	
FR-TO		FROM T	O		LENGTH	H FR-TO			
5-2	-216 / 108	0.0	0.0	0.21 (7)	7.81				
1- 2	0 / 34	-77.3	-77.3	0.11(1)	10.00				
2-3	-56 / 7	-77.3	-77.3	0.06 (7)	6.25				
5- 4	0/0	-17.5	-17.5	0.11 (7)	10.00				

# CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT WIND LOAD TAPPLED IS DERIVED FROM REPERSING VELOCITY FRESSURE OF (\$3.0) FOR AL (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

**RECEIVED** TOWN OF MILTON MAR 29, 2017 17-4978 **BUILDING DIVISION** 

**DESIGN CRITERIA** 

TOTAL WEIGHT = 8 X 9 = 71 lb

SPECIFIED LOADS:

LL = DL = LL = DL = AD = TOP CH. 3.0 PSF 7.0 PSF TOTAL LOAD 33.3

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09

- TPIC 2011

DESIGN ASSUMPTIONS OVERHANG NOT TO BE ALTERED OR CUT

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL)= L/999 (0.00")

CSI: TC=0.21 (2-5:7), BC=0.11 (4-5:7), WB=0.00 (n/a:0), SSI=0.15 (2-5:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

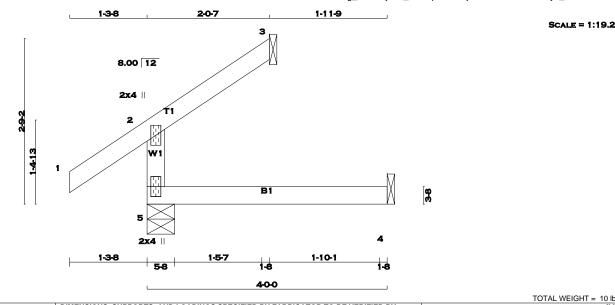
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (5) (INPUT = 0.90) JSI METAL= 0.15 (5) (INPUT = 1.00 )



JOB PR-GREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY PLY DRWG NO. Page 53 of 159 TRUSS DESC. TW0317-048 TW0317-048 J12

Kott Lumber Uxbridge, Stouffville, ON, TW Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:33 2017 Page 1 ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-9itOqaWN3Mjil8BkJobs0?4Mi5IOlyk\_uu17NnzcJJy



LUMBER				
N. L. G. A. F	RULES			
CHORDS	SIZE		LUMBER	DESCR
5 - 2	2x4	DRY	No.2	SPF
1 - 3	2x4	DRY	No.2	SPF
5 - 4	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches) TYPE TMV+p PLATES MT20 W 2.0 LEN Y 4.0 BMV1+p 2.0 4.0

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS

	FACTORED		MAXIMU	M FACT	INPUT	REQRD	
	GROSS R	EACTION	GROSS	REACTIO	BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
5	244	0	257	183	-82	5-8	5-8
3	60	0	71	0	-71	1-8	1-8
4	30	0	34	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) 3, 4

PROVIDE ANCHORAGE AT BEARING JOINT 5 FOR 150 LBS FACTORED PROVIDE ANCHORAGE AT BEARING JOINT 3 FOR 150 LBS FACTORED

PROVIDE FOR 183 LBS FACTORED HORIZONTAL REACTION AT JOINT 5

UNFACTORED REACTIONS											
	1ST LCASE	MAX./	MIN. COMPO	NENT REACTION	ONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL				
5	171	123 / 0	0/0	0/0	33 / -89	48 / 0	0/0				
3	40	36 / 0	0/0	0/0	29 / -54	5/0	0/0				
4	24	0/0	0/0	0/0	4 / -7	24 / 0	0/0				
HORIZONTAL REACTIONS											
5		0/0	0/0	0/0	130 / -88	0/0	0 /0				

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 5. 3

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (11)

CHORDS					WEBS					
MAX.	FACTORED	FACTOR	RED				MAX. FACTO	RED		
MEMB.	FORCE	VERT. LO	AD LC1	1 MAX	MAX.	MEMB.	FORCE	MAX		
	(LBS)	(PLI	F) (	CSI (LC)	UNBRAC		(LBS)	CSI (LC)		
FR-TO		FROM	ΤΌ		LENGTH	FR-TO				
5-2	-219 / 121	0.0	0.0	0.16 (7)	7.81					
1-2	0 / 29	-77.3	-77.3	0.10(1)	10.00					
2-3	-42 / 5	-77.3	-77.3	0.06 (7)	6.25					
5- 4	0/0	-17.5	-17.5	0.06 (7)	10.00					

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

## **DESIGN CRITERIA**

SPECIFIED LOADS: CH. PSF PSF

LL = DL = LL = DL = AD = 3.0 PSF 7.0 PSF TOTAL LOAD 33.3 PSF

## SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

SCALE = 1:19.2

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014

- CSA 086-09 - TPIC 2011

DESIGN ASSUMPTIONS -OVERHANG NOT TO BE ALTERED OR CUT

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19") CALCULATED VERT. DEFL.(LL)= L/ 999 (0.01") ALLOWABLE DEFL.(TL)= L/360 (0.19") CALCULATED VERT. DEFL.(TL)= L/999 (0.01")

CSI: TC=0.16 (2-5:7), BC=0.06 (4-5:7), WB=0.00 (n/a:0), SSI=0.12 (2-5:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (5) (INPUT = 0.90 ) JSI METAL= 0.12 (5) (INPUT = 1.00)





READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

JOB NAME TRUSS NAME

Kott Lumber Uxbridge, Stouffville, ON, TW

TW0317-048

QUANTITY 2 J13

PLY

POBPES GREENPARK-LECCO RIDGE-BLOCK 327

DRWG NO.

Page 54 of 159 TW0317-048

SCALE = 1:26.9

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1-3-8 8.00 12 2x4 || 2 RI 3.50

LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR SIZE 2 2x4 DRY No.2 No.2 SPF SPF DRY No 2

DRY: SEASONED LUMBER

PLATES (table is in inches)

TYPE TMV+p PLATES MT20 W 2.0 LEN Y 4.0 BMV1+p 2.0 4.0

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS

400

	FACTORED		MAXIMU	M FACT	INPUT	REQRD	
	GROSS R	EACTION	GROSS	REACTIO	BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
5	339	0	361	272	-131	5-8	5-8
3	116	0	139	0	-139	1-8	1-8
4	30	0	34	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) 3, 4

PROVIDE ANCHORAGE AT BEARING JOINT 5 FOR 150 LBS FACTORED PROVIDE ANCHORAGE AT BEARING JOINT 3 FOR 150 LBS FACTORED

PROVIDE FOR 272 LBS FACTORED HORIZONTAL REACTION AT JOINT 5

UNFACTORED REACTIONS

	1ST LCASE	MAX./	MIN. COMPON	NENT REACTION	DNS		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
5	235	180 / 0	0/0	0/0	54 / -129	55 / 0	0/0
3	79	70 / 0	0/0	0/0	57 / -105	9/0	0/0
4	24	0/0	0/0	0/0	7 / -11	24 / 0	0/0
HOR 5	IZONTAL REA	ACTIONS 0/0	0/0	0/0	194 / -123	0/0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 5

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (11)

	R D S FACTORED	FACTORE	D			WEI	BS MAX. FACTO	RED	
MEMB.	FORCE	VERT. LOAI	D LC	1 MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PLF)	) (	CSI (LC)	<b>UNBRAC</b>		(LBS)	CSI (LC)	
FR-TO		FROM T	0		LENGTH	FR-TO	, ,	, ,	
5-2	-324 / 176	0.0	0.0	0.25 (7)	7.81				
1-2	0 / 29	-77.3 -	77.3	0.10(1)	10.00				
2-3	-83 / 11	-77.3 -	77.3	0.22 (7)	6.25				
5- 4	0/0	-17.5 -	17.5	0.09 (7)	10.00				

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM},INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

# TOTAL WEIGHT = 2 X 13 = 26 lb **DESIGN CRITERIA**

SPECIFIED LOADS: CH. PSF

LL = DL = LL = DL = AD = 3.0 PSF 7.0 PSF TOTAL LOAD 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014

- CSA 086-09 - TPIC 2011

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19") CALCULATED VERT. DEFL.(LL)= L/ 999 (0.01") ALLOWABLE DEFL.(TL)= L/360 (0.19") CALCULATED VERT. DEFL.(TL)= L/999 (0.01")

CSI: TC=0.25 (2-5:7), BC=0.09 (4-5:7), WB=0.00 (n/a:0), SSI=0.18 (2-5:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.29 (2) (INPUT = 0.90 ) JSI METAL= 0.18 (5) (INPUT = 1.00)





READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

TRUSS NAME JOB NAME

Kott Lumber Uxbridge, Stouffville, ON, TW

J14

TW0317-048

QUANTITY PLY POBPES GREENPARK-LECCO RIDGE-BLOCK 327 TRUSS DESC.

DRWG NO.

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SCALE = 1:26.3

TOTAL WEIGHT = 9 lb [M]

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1-3-8 8.00 12 2x4 || P 3510 3-10-13 3-10-13 W1 **B**1

LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER SIZE DESCR 2 2x4 DRY No.2 No.2 SPF SPF DRY

DRY: SEASONED LUMBER.

PLATES (table is in inches)

TYPE TMV+p PLATES MT20 W 2.0 LEN Y 4.0 BMV1+p 2.0 4.0

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS

200

	FACTOR	ED.	MAXIMU	M FACTO	INPUT	REQRD	
	<b>GROSS RE</b>	ACTION	GROSS F	REACTIO	BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
5	220	0	220	227	-21	5-8	5-8
3	59	0	70	0	-70	1-8	1-8
4	16	0	35	0	-74	1-8	1-8
3 4		0		-			

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) 3, 4

PROVIDE ANCHORAGE AT BEARING JOINT 5 FOR 150 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 3 FOR 150 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 4 FOR 150 LBS FACTORED UPLIFT

PROVIDE FOR 227 LBS FACTORED HORIZONTAL REACTION AT JOINT 5

UNFACTORED REACTIONS

	1ST LCASE	MAX./	MIN. COMPON	<b>IENT REACTIO</b>	ONS		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
5	152	121 / 0	0/0	0/0	0 / -34	30 / 0	0/0
3	40	36 / 0	0/0	0/0	28 / -53	5/0	0/0
4	13	0/0	0/0	0/0	45 / -61	13 / 0	0/0
HOR 5	IZONTAL REA	ACTIONS 0/0	0/0	0/0	162 / -136	0/0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 5

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (12)

W E B S MAX. FACTORED					
MEMB. FORCE MAX					
RAC (LBS) CSI (LC)					
STH FR-TO					
31					
00					
25					
00					

# CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT WIND LOAD TAPPLED IS DERIVED FROM REPERSING VELOCITY FRESSURE OF (\$3.0) FOR AL (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.





READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

**RECEIVED** TOWN OF MILTON MAR 29, 2017 17-4978 **BUILDING DIVISION** 

# **DESIGN CRITERIA**

SPECIFIED LOADS: CH.

LL = DL = LL = DL = AD = PSF 3.0 PSF 7.0 PSF TOTAL LOAD 33.3

## SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014

- CSA 086-09 - TPIC 2011

DESIGN ASSUMPTIONS OVERHANG NOT TO BE ALTERED OR CUT

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL)= L/999 (0.00")

CSI: TC=0.30 (2-5:7), BC=0.20 (4-5:7), WB=0.00 (n/a:0), SSI=0.15 (2-5:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (5) (INPUT = 0.90) JSI METAL= 0.15 (5) (INPUT = 1.00 )

TRUSS NAME JOB NAME

Kott Lumber Uxbridge, Stouffville, ON, TW

TW0317-048

QUANTITY J15

PLY

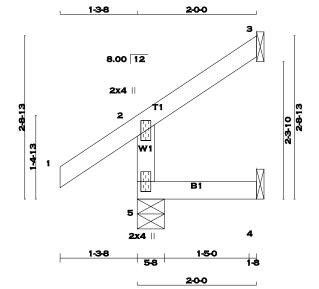
POBPES GREENPARK-LECCO RIDGE-BLOCK 327 TRUSS DESC.

DRWG NO.

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SCALE = 1:19.3

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LUMBER N. L. G. A. RULES CHORDS SIZE

LUMBER SIZE DESCR 2 2x4 DRY No.2 No.2 SPF SPF DRY

DRY: SEASONED LUMBER.

PLATES (table is in inches)

TYPE TMV+p PLATES MT20 W 2.0 LEN Y 4.0 BMV1+p 2.0 4.0

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS

	FACTOR	RED	MAXIMUI	M FACTO	INPUT	REQRD	
	GROSS RE	ACTION	GROSS F	REACTIO	BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
5	222	0	231	181	-78	5-8	5-8
3	59	0	70	0	-70	1-8	1-8
4	16	0	21	0	-16	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) 3, 4

PROVIDE ANCHORAGE AT BEARING JOINT 5 FOR 150 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 3 FOR 150 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 4 FOR 150 LBS FACTORED UPLIFT

PROVIDE FOR 181 LBS FACTORED HORIZONTAL REACTION AT JOINT 5

UNFACTORED REACTIONS

ı		1ST LCASE	MAX./	MIN. COMPON	NENT REACTION	ONS		
ı	JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
ı	5	153	122 / 0	0/0	0/0	25 / -76	31 / 0	0/0
ı	3	40	35 / 0	0/0	0/0	28 / -53	5/0	0/0
ı	4	13	0/0	0/0	0/0	12 / -20	13 / 0	0/0
	HOF 5	RIZONTAL REA	ACTIONS 0/0	0/0	0/0	129 / -88	0/0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 5

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (12)

	R D S FACTORED	FACTOR	ED			W E B	S MAX. FACTO	RED	
MEMB.	FORCE	VERT. LOA	D LC	1 MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PLF	) (	CSI (LC)	UNBRA	С	(LBS)	CSI (LC)	
FR-TO		FROM T	O		LENGTH	HFR-TO	, ,		
5-2	-217 / 120	0.0	0.0	0.15 (7)	7.81				
1- 2	0 / 29	-77.3	-77.3	0.10(1)	10.00				
2-3	-42 / 5	-77.3	-77.3	0.05 (7)	6.25				
5- 4	0/0	-17.5	-17.5	0.07 (7)	10.00				

# CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT WIND LOAD TAPPLED IS DERIVED FROM REPERSING VELOCITY FRESSURE OF (\$3.0) FOR AL (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.





READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

**RECEIVED** TOWN OF MILTON MAR 29, 2017 17-4978 **BUILDING DIVISION** 

## **DESIGN CRITERIA**

SPECIFIED LOADS: CH.

LL = DL = LL = DL = AD = 3.0 PSF 7.0 PSF TOTAL LOAD 33.3

## 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

TOTAL WEIGHT = 4 X 8 = 32 lb

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014

- CSA 086-09 - TPIC 2011

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL)= L/999 (0.00")

CSI: TC=0.15 (2-5:7), BC=0.07 (4-5:7), WB=0.00 (n/a:0), SSI=0.12 (2-5:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (5) (INPUT = 0.90) JSI METAL= 0.12 (5) (INPUT = 1.00 )

TRUSS NAME JOB NAME J16

POBPES GREENPARK-LECCO RIDGE-BLOCK 327 TRUSS DESC.

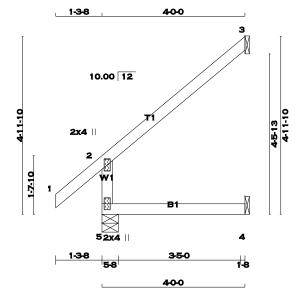
DRWG NO.

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SCALE: 3/8"=1

TW0317-048 Kott Lumber Uxbridge, Stouffville, ON, TW

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LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR SIZE 2 2x4 DRY No.2 No.2 SPF SPF DRY No 2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

TYPE TMV+p PLATES MT20 W 2.0 LEN Y 4.0 BMV1+p 2.0 4.0

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS

	FACTO	RED	MAXIMU	M FACT	INPUT	REQRD	
	GROSS R	EACTION	GROSS	REACTIO	BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
5	340	0	356	338	-97	5-8	5-8
3	116	0	143	0	-165	1-8	1-8
4	31	0	35	0	-4	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) 3, 4

PROVIDE ANCHORAGE AT BEARING JOINT 5 FOR 150 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 3 FOR 165 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 4 FOR 150 LBS FACTORED UPLIFT

PROVIDE FOR 338 LBS FACTORED HORIZONTAL REACTION AT JOINT 5

UNFACTORED REACTIONS

ı		1ST LCASE	MAX./	MAX./MIN. COMPONENT REACTIONS				
١	JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
١	5	236	181 / 0	0/0	0/0	40 / -104	55 / 0	0/0
١	3	79	70 / 0	0/0	0/0	66 / -123	9/0	0/0
١	4	24	0/0	0/0	0/0	11 / -18	24 / 0	0/0
	HOF 5	RIZONTAL REA	ACTIONS 0/0	0/0	0/0	242 / -152	0/0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 5

QUANTITY

5

PLY

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (11)

	R D S FACTORED	FACTORED			WEB:	S AX. FACTO	RED	
MEMB.	FORCE	VERT. LOAD L	C1 MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PLF)	CSI (LC)	<b>UNBRAC</b>	0	(LBS)	CSI (LC)	
FR-TO		FROM TO		LENGTH	FR-TO			
5-2	-321 / 151	0.0 0.	0 0.37 (7)	7.81				
1- 2	0 / 34	-77.3 -77.	3 0.11 (1)	10.00				
2-3	-112 / 15	-77.3 -77.	3 0.26 (7)	6.25				
5- 4	0/0	-17.5 -17.	5 0.14 (7)	10.00				

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT { 40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

## **DESIGN CRITERIA**

SPECIFIED LOADS: CH. PSF

LL = DL = LL = DL = AD = 3.0 PSF 7.0 PSF TOTAL LOAD 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

TOTAL WEIGHT = 5 X 14 = 70 lb

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014

- CSA 086-09 - TPIC 2011

DESIGN ASSUMPTIONS -OVERHANG NOT TO BE ALTERED OR CUT

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL)= L/999 (0.01")

CSI: TC=0.37 (2-5:7), BC=0.14 (4-5:7), WB=0.00 (n/a:0), SSI=0.23 (2-5:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.35 (2) (INPUT = 0.90) JSI METAL= 0.22 (5) (INPUT = 1.00)





READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

JOB NAME TRUSS NAME QUANTITY PLY DRWG NO. TR-GREENPARK-LECCO RIDGE-BLOCK 327 TRUSS DESC TW0317-048 J17 Kott Lumber Uxbridge, Stouffville, ON, TW

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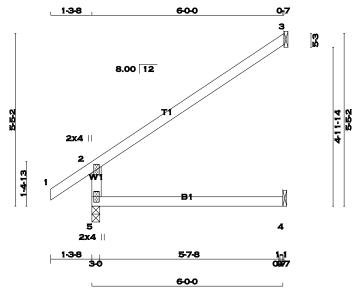
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SCALE = 1:36.2

TOTAL WEIGHT = 18 lb

[M][F

TW0317-048



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR SIZE 2 2x4 DRY No.2 No.2 SPF SPF 2x4 DRY No 2

DRY: SEASONED LUMBER

PLATES (table is in inches) TYPE TMV+p PLATES MT20 W 2.0 LEN Y 4.0 BMV1+p 2.0 4.0

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS

	FACTO GROSS R		MAXIMU GROSS			INPUT	REQRD
	GRUSS R	EACTION	GRUSS	REACTION	JIN	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
5	459	0	491	364	-177	3-0	3-0
3	175	0	209	0	-210	1-8	1-8
4	44	0	49	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) 3, 4

PROVIDE ANCHORAGE AT BEARING JOINT 5 FOR 177 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 3 FOR 210 LBS FACTORED UPLIFT

PROVIDE FOR 364 LBS FACTORED HORIZONTAL REACTION AT JOINT 5

UNFACTORED REACTIONS 1ST LCASE COMBINED MAX./MIN. COMPONENT REACTIONS SNOW LIVE PERM.LIVE V JΤ WIND SOIL DEAD 319 119 0/0 0/0 81 / -178 86 / -159 80 / 0 14 / 0 0/0 240 / 0 35 0/0 0/0 0/0 5/-8 35 / 0 0/0 HORIZONTAL REACTIONS 0/0 0/0 0/0 260 / -158 0/0 0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 5

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (11)

CHORDS WEBS MAX. FACTORED MAX. FACTORED FACTORED MEMB. VERT. LOAD LC1 MAX MAX. МЕМВ. FORCE CSI (LC) UNBRAC (LBS) (PLF) (LBS) CSI (LC) FR-TO FROM TO LENGTH FR-TO 0.0 0.0 0.35 (7) -77.3 -77.3 0.10 (1) -77.3 -77.3 0.39 (7) 5- 2 1- 2 2- 3 -432 / 232 0/29 10.00 -126 / 16 5-4 -17.5 -17.5 0.13 (11) 10.00 0/0

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {440-0, FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CPCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.

# **DESIGN CRITERIA**

SPECIFIED LOADS: LL = DL = LL = DL = AD = CH. PSF 3.0 PSF 7.0 PSF TOTAL LOAD 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014

- CSA 086-09 - TPIC 2011

DESIGN ASSUMPTIONS -OVERHANG NOT TO BE ALTERED OR CUT

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20") CALCULATED VERT. DEFL.(LL)= L/ 999 (0.02") ALLOWABLE DEFL.(TL)= L/360 (0.20") CALCULATED VERT. DEFL.(TL)= L/999 (0.04")

CSI: TC=0.39 (2-3:7), BC=0.13 (4-5:11), WB=0.00 (n/a:0), SSI=0.25 (2-5:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.40 (2) (INPUT = 0.90) JSI METAL= 0.24 (5) (INPUT = 1.00)





READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

JOB PESC TR-GREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY PLY TRUSS DESC. TW0317-048 T01 Kott Lumber Uxbridge, Stouffville, ON, TW

840

1-3-8

DRWG NO.

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SCALE = 1:50.2

TOTAL WEIGHT = 88 lb

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1-3-8

8.00 12 3x5 🗸 3x5 < 5 STЗ ST3 ST4 WЗ 3x6 🗸 3x6 < W2 12 10 11 4x5 4x5 || 3x6 = 13 9 4x4 || 4x4 || 1-3-8 1590 1-3-8 4-2-8 4-2-8 830

LUMBER				
N. L. G. A. R	ULES			
CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 7	2x4	DRY	No.2	SPF
14 - 2	2x4	DRY	No.2	SPF
8 - 6	2x4	DRY	No.2	SPF
14 - 13	2x4	DRY	No.2	SPF
13 - 3	2x4	DRY	No.2	SPF
12 - 10	2x4	DRY	No.2	SPF
9 - 5	2x4	DRY	No.2	SPF
9 - 8	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
ALL CABLE	WEDE			

DRY 2x3 No.2 DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

	LATEO (tubic is in inches)						
JT	TYPE	PLATES	W	LEN	Υ	X	
2	TMVW-t	MT20	3.0	6.0			
3	TMVW-t	MT20	3.0	5.0	1.50	2.00	
3	NP+w	MT20	2.0	4.0	1.75	0.75	
4	TTW+p	MT20	3.0	4.0	2.25	1.50	
5	TMVW-t	MT20	3.0	5.0	1.50	2.00	
5	NP+w	MT20	2.0	4.0	1.75	0.75	
6	TMVW-t	MT20	3.0	6.0			
8	BMV1+p	MT20	2.0	4.0			
9	BMVW+p	MT20	4.0	4.0	1.50	1.75	
10,	12, 18, 24						
10							
11	BMWWW-t	MT20	3.0	6.0			
12		MT20	4.0	5.0		2.00	
13	BMVW+p	MT20	4.0	4.0	1.50	1.75	
14	BMV1+p	MT20	2.0	4.0			
15	NP-p	MT20	2.0	4.0	0.75	2.00	
15,	16, 17, 19, 20	), 21, 22, 23	, 25, 26				
15	NP+w	MT20	2.0	4.0			
23	NP-p	MT20	2.0	4.0	0.75	2.00	
24	BVMW+p	MT20	4.0	5.0	Edge	1.75	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.



DIMENSIONS, SUPPORTS	AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY
BUILDING DESIGNER	

BEA	RINGS						
	FACTO	RED	MAXIMU	M FACT	ORED	INPUT	REQRD
	GROSS R	EACTION	GROSS	REACTIO	N	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
14	897	0	950	-316	-392	5-8	5-8
8	897	0	950	0	-392	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 14 FOR 392 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 8 FOR 392 LBS FACTORED UPLIFT

PROVIDE FOR 316 LBS FACTORED HORIZONTAL REACTION AT JOINT 14

ALLOW FOR 0.4" OF HORIZONTAL MOVEMENT DUE TO TOTAL LOAD

UNFACTORED REACTIONS

SOIL
0/0
0/0
0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 14, 8

SPF

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (11)

	RDS					WE		
MAX.	FACTORED	FACTO	RED				MAX. FACTO	DRED
MEMB.	FORCE	VERT. LO	AD LC1	1 MAX	MAX.	MEMB.	FORCE	MAX
	(LBS)	(PL	.F) (	CSI (LC)	UNBRAC		(LBS)	CSI (LC)
FR-TO			TO		LENGTH	FR-TO		
1- 2	0 / 29	-77.3	-77.3	0.10(1)	10.00	11- 4	-264 / 541	0.14 (7)
2-3	-834 / 375	-77.3	-77.3	0.25 (7)	6.25	11- 5	-441 / 379	0.18 (4)
3-4	-768 / 375	-77.3	-77.3	0.25 (7)	6.25	3-11	-457 / 408	0.19 (3)
4- 5	-783 / 408	-77.3	-77.3	0.26 (8)	6.25	2-13	-222 / 732	0.15 (1)
5-6	-836 / 379	-77.3	-77.3	0.27 (8)	6.25	9-6	-217 / 730	0.15 (1)
6- 7	0 / 29	-77.3	-77.3	0.10(1)	10.00			
14-2	-914 / 417	0.0	0.0	0.09(1)	7.81			
8-6	-914 / 417	0.0	0.0	0.09 (1)	7.81			
4440	005 / 000	47.5	47.5	0.00 (44)	0.05			
	-295 / 306			0.08 (11)				
13-12	-153 / 83			0.90 (1)				
12-3	-124 / 103	0.0	0.0	0.91 (1)	7.81			
12-11	-432 / 1035	-17.5	-17.5	0.19(1)	6.25			
11-10	-256 / 958	-17.5	-17.5	0.19(1)	6.25			
9-10	-152 / 81	0.0	0.0	0.90(1)	7.81			
10-5	-123 / 101	0.0	0.0	0.91 (1)	7.81			
9-8	-10 / 21	-17.5		0.08 (11)				

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT  $\{40\text{-}0\}$  FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM,INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

RECEIVED TOWN OF MILTON MAR 29, 2017 17-4978 **BUILDING DIVISION** 

# **DESIGN CRITERIA**

SPECIFIED LOADS: LL = DL = LL = DL = AD = CH. PSF 3.0 PSF 7.0 PSF TOTAL LOAD 33.3 PSF

## SPACING = 24.0 IN. C/C

- TPIC 2011

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012, BCBC 2012, ABC 2014 - CSA 086-09

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.56") CALCULATED VERT. DEFL.(LL) = L/999 (0.15") ALLOWABLE DEFL.(TL)= L/360 (0.56") CALCULATED VERT. DEFL.(TL) = L/815 (0.25")

CSI: TC=0.27 (5-6:8) , BC=0.91 (3-12:1) , WB=0.19 (3-11:3) , SSI=0.52 (12-13:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (13) (INPUT = 0.90 ) JSI METAL= 0.25 (2) (INPUT = 1.00 )

JOB PESC TR-GREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY PLY TRUSS DESC. TW0317-048 T02

840

1-3-8

DRWG NO

Page 60 of 159 TW0317-048

SCALE = 1:50.3

TOTAL WEIGHT = 74 lb

Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:35 2017 Page ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-54?8EGXebzzPXSL6RDeK5Q9fWumiDpKHMCWESfzcJJw

1-3-8

3x4 II 8.00 12 3x5 / 3x5 > 5 TJ 3x6 🗸 3x6 < W2 W2 車 12 10 BI 11 BI 1 8 **3x4** = 3x4 3x6 = 13 2x4 | 4x4 || 2x4 || 4x4 1590 1.3-8 58 428 428 830

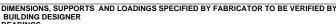
LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR 4 DRY No.2 No.2 SPF SPF 2 6 13 14 -2x4 DRY No 2 SPF 8 -14-SPF 2x4 No.2 2x4 DRY No.2 13 -12 -3 DRY DRY SPF SPF No.2 2x4 No.2 SPF SPF 2v4 DRY No.2 No.2 ALL WEBS EXCEPT 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

Kott Lumber Uxbridge, Stouffville, ON, TW

PL	PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Υ	X	
2	TMVW-t	MT20	3.0	6.0			
3	TMVW-t	MT20	3.0	5.0	1.50	2.00	
4	TTW+p	MT20	3.0	4.0	2.25	1.50	
5	TMVW-t	MT20	3.0	5.0	1.50	2.00	
6	TMVW-t	MT20	3.0	6.0			
8	BMV1+p	MT20	2.0	4.0			
9	BMVW+p	MT20	4.0	4.0	1.50	1.75	
10	BVM-I	MT20	3.0	4.0			
11	BMWWW-t	MT20	3.0	6.0			
12	BVM-I	MT20	3.0	4.0			
13	BMVW+p	MT20	4.0	4.0	1.50	1.75	
1/	RM\/1±n	MT20	2.0	4.0			

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.



BEAL	RINGS						
	FACTOR	ED	MAXIMUM FACTORED			INPUT	REQRD
	<b>GROSS RE</b>	ACTION	GROSS F	REACTIO	N	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
14	897	0	950	-316	-392	5-8	5-8
8	897	0	950	0	-392	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 14 FOR 392 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 8 FOR 392 LBS FACTORED UPLIFT

PROVIDE FOR 316 LBS FACTORED HORIZONTAL REACTION AT JOINT 14

ALLOW FOR 0.4" OF HORIZONTAL MOVEMENT DUE TO TOTAL LOAD

<b>UNFACTORED REAC</b>	TIONS	
40T L 0 4 0 F	B 4 6 3 / /B 415 I	COMPONENT DE COTIONIO

1ST LCASE	MAX./I	MAX./MIN. COMPONENT REACTIONS					
COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
627	452 / 0	0/0	0/0	133 / -392	175 / 0	0/0	
627	452 / 0	0/0	0/0	133 / -392	175 / 0	0/0	
IZONTAL RE	ACTIONS						
	0/0	0/0	0/0	226 / -226	0/0	0 /0	
	COMBINED 627 627 IZONTAL RE	COMBINED SNOW 627 452 / 0 627 452 / 0 IZONTAL REACTIONS	COMBINED SNOW LIVE 627 452 / 0 0 / 0 627 452 / 0 0 / 0 IZONTAL REACTIONS	COMBINED SNOW LIVE PERM.LIVE 627 452 / 0 0 / 0 0 / 0 627 452 / 0 0 / 0 0 / 0 0 / 0 IZONTAL REACTIONS	COMBINED SNOW LIVE PERM.LIVE WIND 627 452/0 0/0 0/0 133/-392 627 452/0 0/0 0/0 133/-392 IZONTAL REACTIONS	COMBINED         SNOW         LIVE         PERM.LIVE         WIND         DEAD           627         452 / 0         0 / 0         0 / 0         133 / -392         175 / 0           627         452 / 0         0 / 0         0 / 0         133 / -392         175 / 0           IZONTAL REACTIONS	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 14. 8

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (11)

	RDS	FAOTO				WE		
	FACTORED						MAX. FACTO	
MEMB.	FORCE	VERT. LO	AD LC	1 MAX	MAX.	MEMB.	FORCE	MAX
	(LBS)	(PL	.F) (	CSI (LC)	<b>UNBRAC</b>	0	(LBS)	CSI (LC)
FR-TO	. ,	FROM			LENGTH		. ,	, ,
1- 2	0/29	-77.3	-77.3	0.10(1)	10.00	11- 4	-264 / 541	0.14 (7)
2-3	-834 / 375	-77.3	-77.3	0.25 (7)	6.25	11- 5	-441 / 379	0.18 (4)
3- 4	-768 / 375	-77.3	-77.3	0.25 (7)	6.25	3-11	-457 / 408	0.19 (3)
4- 5	-783 / 408	-77.3	-77.3	0.26 (8)	6.25	2-13	-222 / 732	0.15 (1)
5-6	-836 / 379	-77.3	-77.3	0.27 (8)	6.25	9-6	-217 / 730	0.15 (1)
6- 7	0 / 29	-77.3	-77.3	0.10(1)	10.00			
14- 2	-914 / 417	0.0	0.0	0.09(1)	7.81			
8- 6	-914 / 417	0.0	0.0	0.09(1)	7.81			
14-13	-295 / 306	-17.5	-17.5	0.08 (11)	6.25			
13-12	-153 / 83	0.0	0.0	0.90(1)	7.81			
12-3	-124 / 103	0.0	0.0	0.91(1)	7.81			
12-11	-432 / 1035	-17.5	-17.5	0.19(1)	6.25			
11-10	-256 / 958	-17.5	-17.5	0.19(1)	6.25			
9-10	-152 / 81	0.0	0.0	0.90(1)	7.81			
10-5	-123 / 101	0.0	0.0	0.91 (1)	7.81			
9-8	-10 / 21	-17.5	-17.5	0.08 (11)	10.00			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT  $\{40\text{-}0\}$  FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM,INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

**RECEIVED** TOWN OF MILTON MAR 29, 2017 17-4978 **BUILDING DIVISION** 

# **DESIGN CRITERIA**

SPEC	IFIED	LOAI	DS:		
TOP	CH.	LL	=	23.3	PSF
		DL	=	3.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.0	PSF
TOTA	L LO	AD	=	33.3	PSF

## SPACING = 24.0

TPIC 2011

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014 - CSA 086-09

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.56") CALCULATED VERT. DEFL.(LL) = L/999 (0.15") ALLOWABLE DEFL.(TL)= L/360 (0.56") CALCULATED VERT. DEFL.(TL) = L/815 (0.25")

CSI: TC=0.27 (5-6:8), BC=0.91 (3-12:1), WB=0.19 (3-11:3) , SSI=0.52 (12-13:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (13) (INPUT = 0.90 ) JSI METAL= 0.25 (12) (INPUT = 1.00 )



POBPES GREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY DRWG NO. Page 61 of 159 TW0317-048 2 TW0317-048 T03 Kott Lumber Uxbridge, Stouffville, ON, TW Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:36 2017 Page ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-ZHZXScYGMH5G9cvJ?w9ZedimwlF3yBoQbsGn\_6zcJJy 5-7-12 16-1-7 5-1-12 1.3-8 SCALE = 1:46.4 5x6 \\ 2x4 || 3x6 = 3x4 = 5x6 // 3 6 5 8.00 12 4x5 < 4x5 / 8 12 14 13 11 10 15 **3**x6 = 3x8 = 3x8 = 3x5 = 2x4 | 4x4 =2x4 || 1-3-8 26-5-8

LUMBER				
N. L. G. A. F	RULES			
CHORDS	SIZE		LUMBER	DESCR.
1 - 3	2x4	DRY	No.2	SPF
3 - 5	2x4	DRY	No.2	SPF
5 - 7	2x4	DRY	No.2	SPF
7 - 8	2x4	DRY	No.2	SPF
15 - 2	2x4	DRY	No.2	SPF
9 - 8	2x4	DRY	No.2	SPF
15 - 12	2x4	DRY	No.2	SPF
12 - 9	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PL/	PLATES (table is in inches)								
JT	TYPE	PLATES	W	LEN	Υ	X			
2	TMVW-t	MT20	4.0	5.0	1.50	2.00			
3	TTWW+m	MT20	5.0	6.0	Edge	3.75			
4	TMW+w	MT20	2.0	4.0					
5	TS-t	MT20	3.0	6.0					
6	TMWW-t	MT20	3.0	4.0					
7	TTWW+m	MT20	5.0	6.0	Edge	3.75			
8	TMVW-t	MT20	4.0	5.0	1.75	Edge			
9	BMV1+p	MT20	2.0	4.0					
10	BMWW-t	MT20	3.0	5.0	1.50	1.75			
11	BMWW-t	MT20	4.0	4.0					
12	BS-t	MT20	3.0	6.0					
13	BMWWW-t	MT20	3.0	8.0					
14	BMWW-t	MT20	3.0	8.0	1.50	3.00			
15	BMV1+p	MT20	2.0	4.0					

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY
BUILDING DESIGNER
BEARINGS
BEARINGS

27-0-10

3EA	RINGS						
	FACTOR	RED	MAXIMUI	M FACTO	ORED	INPUT	REQRD
	GROSS RE	ACTION	GROSS F	REACTIO	N	BRG	BRG
ΙT	VERT	HORZ	DOWN	HORZ	<b>UPLIFT</b>	IN-SX	IN-SX
15	1383	0	1467	241	-712	5-8	5-8
9	1276	0	1318	0	-648	1-10	1-10

PROVIDE ANCHORAGE AT BEARING JOINT 15 FOR 712 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 9 FOR 648 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES

PROVIDE FOR 241 LBS FACTORED HORIZONTAL REACTION AT JOINT 15

UNF	UNFACTORED REACTIONS							
	1ST LCASE	MAX./	MIN. COMPO	NENT REACTION	ONS			
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
15	968	691 / 0	0/0	0/0	211 / -687	277 / 0	0/0	
9	896	627 / 0	0/0	0/0	103 / -636	269 / 0	0/0	
HOR 15	HORIZONTAL REACTIONS 15 0/0 0/0 0/0 172/-158 0/0 0/0							
BEAF	RING MATER	IAL TO BE S	SPF NO.2 OR	BETTER AT JO	DINT(S) 15, 9			

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.63 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (11)

	ORDS X. FACTORED	FACTOR	RED			WE	B S MAX. FACTO	ORED	
MEMB.		VERT. LO							
IVILIVID.		(PL							
FR-TO		FROM					(LDO)	001 (L0)	
	0 / 29						-118 / 165	0.04 (3)	
	-1506 / 753							0.45 (8)	
	-1851 / 1042						-471 / 364	0.18 (3)	
4- 5	-1851 / 1043			0.37 (1)			-65 / 56	0.06 (6)	
	-1851 / 1043			0.37 (1)			-508 / 403	0.19 (3)	
	-1814 / 1021						-505 / 895		
	-1389 / 708						-206 / 197		
15- 2	-1426 / 741	0.0	0.0	0.14(1)	6.88	2-14	-467 / 1241	0.27 (1)	
9-8	-1281 / 674	0.0	0.0	0.13(1)	7.17	10-8	-474 / 1196	0.26 (1)	
15-14	-220 / 212	-17.5	-17.5	0.13 (11	) 6.25				
14-13	-552 / 1272	-17.5	-17.5	0.26 (1)	6.25				
13-12	-801 / 1816	-17.5	-17.5	0.32 (1)	6.25				
12-11	-801 / 1816	-17.5	-17.5	0.32(1)	6.25				
11-10	-426 / 1151	-17.5	-17.5	0.24(1)	6.25				
10-9	-13 / 28	-17.5	-17.5	0.12 (11	) 6.25				

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CPCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

**RECEIVED** TOWN OF MILTON MAR 29, 2017 17-4978 **BUILDING DIVISION** 

# TOTAL WEIGHT = 2 X 109 = 218 lb **DESIGN CRITERIA**

SPECIFIED LOADS: TOP CH. LL = LL = 23.3 DL = 3.0 LL = 0.0 DL = 7.0 AD = 33.3 PSF TOTAL LOAD

## SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

1-10

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09

- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.90") CALCULATED VERT. DEFL.(LL)= L/ 999 (0.08") ALLOWABLE DEFL.(TL)= L/360 (0.90") CALCULATED VERT. DEFL.(TL)= L/ 999 (0.14")

CSI: TC=0.54 (2-3:7) , BC=0.32 (11-13:1) , WB=0.49 (7-11:7) , SSI=0.19 (6-7:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

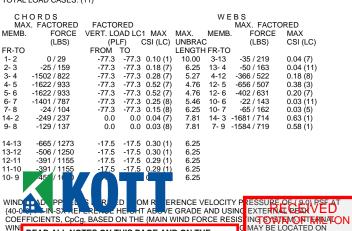
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (10) (INPUT = 0.90 ) JSI METAL= 0.51 (2) (INPUT = 1.00 )



JOB NAME TRUSS NAME QUANTITY DRWG NO. TR-GREENPARK-LECCO RIDGE-BLOCK 327 Page 62 of 159 2 TW0317-048 TW0317-048 T04 Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:36 2017 Page Kott Lumber Uxbridge, Stouffville, ON, TW ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-ZHZXScYGMH5G9cvJ?w9ZedimJIGLy9cQbsGn\_6zcJJy 7-1-12 13-1-7 6-7-12 1.3-8 SCALE = 1:48.5 5x5 \\ 5x5 // 2x4 | 4 5 6 8.00 12 4x4 / 3x6 < 3 W5 71 🙉 2x4 || 2x4 || **B2** FH: 11 13 12 10 3x6 = 4x4 = 4x4 3x4 = 3x8 = 3x4 = 1-3-8 2658 5-8 1-10 27-0-10 TOTAL WEIGHT = 2 X 112 = 224 I LUMBER N. L. G. A. CHORDS DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER **DESIGN CRITERIA** LUMBER DESCR SIZE BEARINGS FACTORED DRY SPF SPF MAXIMUM FACTORED INPLIT REQRD SPECIFIED LOADS: LL = DL = LL = DL = AD = No.2 GROSS REACTION GROSS REACTION TOP CH. LIPLIFT IN-SX 3.0 2x4 DRY No 2 VFRT HOR7 DOWN HORZ IN-SX PSF 2x4 No.2 14 9 8 2x4 DRY No.2 SPF 1319 -634 1-10 1-10 7.0 **PSF** TOTAL LOAD 33.3 SPF PROVIDE ANCHORAGE AT BEARING JOINT 14 FOR 698 LBS FACTORED 9 No.2 PROVIDE ANCHORAGE AT BEARING JOINT 9 FOR 634 LBS. FACTORED SPACING = 24.0 IN. C/C ALL WEBS EXCEPT NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12 DRY: SEASONED LUMBER. PROVIDE FOR 281 LBS FACTORED HORIZONTAL REACTION AT JOINT 14 THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF UNFACTORED REACTIONS 1ST LCASE COMBINED MAX./MIN. COMPONENT REACTIONS
SNOW LIVE PERM.LIVE WIND PART 9, NBCC 2010 PLATES (table is in inches) DEAD SOIL 220 / -677 0/0 PLATES W LEN Y 691 / 0 0/0 0/0 THIS DESIGN COMPLIES WITH: 4.0 - PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 TMWW-t MT20 4.0 1.50 1.75 - CSA 086-09 TTWW+m MT20 2.00 1.50 HORIZONTAL REACTIONS - TPIC 2011 0/0 0/0 0/0 MT20 200 / -186 0 /0 4.0 0/0 5.0 3.0 2.0 5.0 6.0 (55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED TTWW+m MT20 2 25 1 25 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 14, 9 4.0 a+VMT MT20 ROOF LIVE LOAD 4.0 BMVW1-MT20 4.0 1.50 1.75 ALLOWABLE DEFL.(LL)= L/360 (0.90") CALCULATED VERT. DEFL.(LL)= L/ 999 (0.06") ALLOWABLE DEFL.(TL)= L/360 (0.90") CALCULATED VERT. DEFL.(TL)= L/999 (0.11") TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.76 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY BMWW-t MT20 3.0 BS-t MT20 6.0 BMWWW-13 BMWW-t MT20 3.0 4.0 BMVW1-t MT20 4.0 4.0 1.50 1.75 ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED. CSI: TC=0.52 (4-5:7) , BC=0.30 (12-13:1) , WB=0.63 (3-14:1) , SSI=0.25 (4-5:1) 1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 4-12, 6-12. DBS = 20-0-0 . CBF = 44 A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12. COMP=1.10 SHEAR=1.10 TENS= 1.10 COMPANION LIVE LOAD FACTOR = 0.50 END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE <u>LOADING</u> TOTAL LOAD CASES: (11) TRUSS MANUFACTURING PLANT NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN CHORDS MAX. FACTORED WEBS MAX. FACTORED **FACTORED** VERT, LOAD LC1 MAX MAX. MEMB FORCE MEMB FORCE MAX 618 354 1667 822 2284 1656



17-4978 **BUILDING DIVISION** 

READ ALL NOTES ON THIS PAGE AND ON THE

IN THE DESIGN OF THIS COMPONENT.

ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE

IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED

TL. WISE THE

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March 10, 2017

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OF

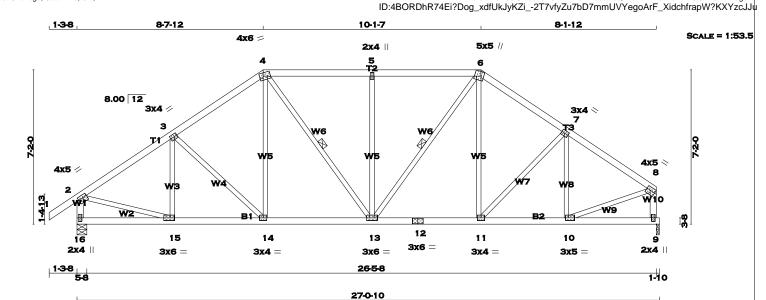
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (14) (INPUT = 0.90)

JSI METAL= 0.62 (3) (INPUT = 1.00)

POBPES GREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY DRWG NO. Page 63 of 159 2 TW0317-048 TW0317-048 T05 Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:37 2017 Page Kott Lumber Uxbridge, Stouffville, ON, TW



LUMBER				
N. L. G. A. R	ULES			
CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 6	2x4	DRY	No.2	SPF
6 - 8	2x4	DRY	No.2	SPF
16 - 2	2x4	DRY	No.2	SPF
9 - 8	2x4	DRY	No.2	SPF
16 - 12	2x4	DRY	No.2	SPF
12 - 9	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)								
JT	TYPE	PLATES	W	LEN	Υ	X		
2	TMVW-t	MT20	4.0	5.0	1.50	2.00		
3	TMWW-t	MT20	3.0	4.0	1.50	1.50		
4	TTWW-m	MT20	4.0	6.0	1.75	2.00		
5	TMW+w	MT20	2.0	4.0				
6	TTWW+m	MT20	5.0	5.0	2.50	1.50		
7	TMWW-t	MT20	3.0	4.0	1.50	1.50		
8	TMVW-t	MT20	4.0	5.0	1.75	Edge		
9	BMV1+p	MT20	2.0	4.0				
10	BMWW-t	MT20	3.0	5.0	1.50	1.75		
11	BMWW-t	MT20	3.0	4.0				
12	BS-t	MT20	3.0	6.0				
13	BMWWW-t	MT20	3.0	6.0				
14	BMWW-t	MT20	3.0	4.0				
15	BMWW-t	MT20	3.0	6.0	1.50	2.50		
16	BMV1+p	MT20	2.0	4.0				

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH

TEE-LOK TL20 PLATES IS ALLOWED.

T.L. WISE

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March 10, 2017

DIMENSIONS, SUPPORTS	AND LOADINGS SPECII	FIED BY FABRICATO	R TO BE VERIFIED BY
BUILDING DESIGNER			
BEARINGS			

	FACTORED		MAXIMU	M FACT	INPUT	REQRD	
	GROSS REACTION GROSS REACTION				BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
16	1383	0	1473	320	-681	5-8	5-8
9	1276	0	1330	0	-616	1-10	1-10

PROVIDE ANCHORAGE AT BEARING JOINT 16 FOR 681 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 9 FOR 616 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES

PROVIDE FOR 320 LBS FACTORED HORIZONTAL REACTION AT JOINT 16

UNFACTORED REACT	IONS	
10T L CACE	MANV	/h /

	1ST LCASE	MAX./	<u>MIN. COMPOR</u>	<u>NENT REACTIO</u>	ONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
16	968	691 / 0	0/0	0/0	225 / -665	277 / 0	0/0		
9	896	627 / 0	0/0	0/0	133 / -613	269 / 0	0/0		
HOR	HORIZONTAL REACTIONS								
16		0/0	0/0	0/0	229 / -214	0/0	0 /0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 16, 9

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.15 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 4-13, 6-13. DBS = 20-0-0 . CBF = 31

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING TOTAL LOAD CASES: (11)

	ORDS					WE			
	(. FACTORED	FACTO					MAX. FACTO		
MEMB.	FORCE	VERT. LO	AD LC	1 MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PL	.F) (	CSI (LC)	UNBRAC	2	(LBS)	CSI (LC)	
FR-TO		FROM	TO		LENGTH	FR-TO			
1- 2	0 / 29	-77.3	-77.3	0.10(1)	10.00	15-3	-233 / 182	0.06(1)	
2-3	-1539 / 749	-77.3	-77.3	0.35 (7)	5.15	3-14	-226 / 273	0.13(3)	
3-4	-1430 / 800	-77.3	-77.3	0.36 (7)	5.29	14- 4	-134 / 238	0.12 (7)	
4- 5	-1365 / 807	-77.3	-77.3	0.34 (7)	5.33	4-13	-254 / 345	0.12 (8)	
5-6	-1365 / 807	-77.3	-77.3	0.34 (7)	5.33	13- 5	-502 / 388	0.44 (3)	
6- 7	-1357 / 777	-77.3	-77.3	0.33 (8)	5.44	13-6	-284 / 427	0.13 (7)	
7-8	-1370 / 682	-77.3	-77.3	0.32 (8)	5.40	11-6	-103 / 178	0.09 (8)	
16- 2	-1438 / 702	0.0	0.0	0.14(1)	6.85	11-7	-134 / 215	0.08 (4)	
9-8	-1297 / 636	0.0	0.0	0.13(1)	7.12	10-7	-317 / 219	0.09(1)	
				. ,		2-15	-489 / 1311	0.28 (1)	
16-15	-299 / 290	-17.5	-17.5	0.07 (11	) 6.25	10-8	-491 / 1219	0.26 (1)	
15-14	-654 / 1350	-17.5	-17.5	0.24 (1)	6.25				
14-13	-456 / 1189	-17.5	-17.5	0.22(1)	6.25				
13-12_	-355 / 1115	-17.5	-17.5	0.21 (1)	6.25				
12-11	35 / 1			(	6.25				
11-10	/1	17	- 5 - 5	0.2 1)	6.25				
10-9	/ 28	17	- 5	0.0 11	) 6.25				
								EOEN/	云
							K	ECEIVE	:ບ
WIND L	OAD APPLIED	IS DERIVE	D FRO	M REFER	RENCE V	ELOCITY	Y PRESIONA	/INF(NOF-DIAPE	FTA
(40							CVTEDN		

READ ALL NOTES ON THIS PAGE AND ON THE COE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE WIN IS AN INTEGRAL PART OF THIS DRAWING AS IT

CONTAINS SPECIFICATIONS AND CRITERIA USED

IN THE DESIGN OF THIS COMPONENT.

NO EXTERNAL PEAK ING S**YMAFRI) MOTERNAL** MAY BE LOCATED ON EAST (0-0) #**7**-1**4-3** X **3** WAY

**BUILDING DIVISION** 

TOTAL WEIGHT = 2 X 121 = 243 lb

**DESIGN CRITERIA** SPECIFIED LOADS:

LL = 23.3 DL = 3.0 LL = 0.0 DL = 7.0 AD = 33.3 CH. PSF TOTAL LOAD

## SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09

- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.90")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.90")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.09")

CSI: TC=0.36 (3-4:7) , BC=0.24 (14-15:1) , WB=0.44 (5-13:3) , SSI=0.19 (4-5:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

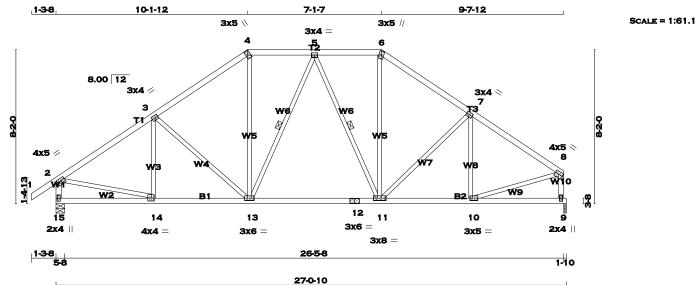
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (10) (INPUT = 0.90 ) JSI METAL= 0.51 (2) (INPUT = 1.00 )

TRUSS NAME POBPES GREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME QUANTITY DRWG NO. Page 64 of 159 TRUSS DESC. TW0317-048 TW0317-048 T06 Kott Lumber Uxbridge, Stouffville, ON, TW

Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:37 2017 Page ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-2T7vfyZu7bD7mmUVYegoArFy7ic?hh3apW?KXYzcJJu 9-7-12



LUMBER									
N. L. G. A. R	N. L. G. A. RULES								
CHORDS	SIZE		LUMBER	DESCR.					
1 - 4	2x4	DRY	No.2	SPF					
4 - 6	2x4	DRY	No.2	SPF					
6 - 8	2x4	DRY	No.2	SPF					
15 - 2	2x4	DRY	No.2	SPF					
9 - 8	2x4	DRY	No.2	SPF					
15 - 12	2x4	DRY	No.2	SPF					
12 - 9	2x4	DRY	No.2	SPF					
ALL WEBS	2x3	DRY	No.2	SPF					
EXCEPT									

DRY: SEASONED LUMBER.

PL/	PLATES (table is in inches)								
JT	TYPE	PLATES	· W	LEN	Υ	X			
2	TMVW-t	MT20	4.0	5.0	1.50	2.00			
3	TMWW-t	MT20	3.0	4.0	1.50	1.50			
4	TTW+h	MT20	3.0	5.0	2.50	1.00			
5	TMWW-t	MT20	3.0	4.0					
6	TTW+m	MT20	3.0	5.0	2.50	1.25			
7	TMWW-t	MT20	3.0	4.0	1.50	1.50			
8	TMVW-t	MT20	4.0	5.0	1.75	Edge			
9	BMV1+p	MT20	2.0	4.0					
10	BMWW-t	MT20	3.0	5.0	1.50	1.75			
11	BMWWW-t	MT20	3.0	8.0					
12	BS-t	MT20	3.0	6.0					
13	BMWWW-t	MT20	3.0	6.0					
14	BMWW-t	MT20	4.0	4.0	1.75	1.50			
15	BMV1+p	MT20	2.0	4.0					

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS	AND LOADINGS	SPECIFIED BY	FABRICATOR 1	TO BE VERIFIED	BY
BUILDING DESIGNER					
TARINOC					

3EA	EARINGS									
	FACTORED		MAXIMUM FACTORED			INPUT	REQRD			
	GROSS REACTION GROSS RE				EACTION BR		BRG			
JΤ	VERT	HORZ	DOWN	HORZ	<b>UPLIFT</b>	IN-SX	IN-SX			
15	1383	0	1474	359	-661	5-8	5-8			
9	1276	0	1339	0	-595	1-10	1-10			

PROVIDE ANCHORAGE AT BEARING JOINT 15 FOR 661 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 9 FOR 595 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES

PROVIDE FOR 359 LBS FACTORED HORIZONTAL REACTION AT JOINT 15

UNF	INFACTORED REACTIONS									
	1ST LCASE	MAX./	MAX./MIN. COMPONENT REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL			
15	968	691 / 0	0/0	0/0	228 / -650	277 / 0	0/0			
9	896	627 / 0	0/0	0/0	156 / -598	269 / 0	0/0			
HOR 15	IZONTAL REA	ACTIONS 0/0	0/0	0/0	257 / -242	0/0	0 /0			

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 15, 9

D

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.02 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 5-13, 5-11. DBS = 20-0-0 . CBF = 27

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING TOTAL LOAD CASES: (11)

СН	ORDS					WE	BS		
MA)	K. FACTORED	FACTO	RED				MAX. FACTO	DRED	
MEMB.	FORCE	VERT. LO			MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PL	.F) (	CSI (LC)	<b>UNBRAC</b>	2	(LBS)	CSI (LC)	
FR-TO		FROM	TO		LENGTH	FR-TO			
1-2	0 / 29	-77.3	-77.3	0.10(1)	10.00	14- 3	-180 / 164	0.06(3)	
2-3	-1557 / 731	-77.3	-77.3	0.45 (7)	5.02	3-13	-348 / 365	0.29 (3)	
3-4	-1352 / 746	-77.3	-77.3	0.45 (7)	5.31	13- 4	-232 / 456	0.30 (8)	
4-5	-1121 / 709	-77.3	-77.3	0.21 (7)	5.91	13- 5	-185 / 218	0.09 (4)	
5-6	-1072 / 693	-77.3	-77.3	0.20 (8)	6.01	5-11	-252 / 244	0.12 (3)	
6-7	-1306 / 732	-77.3	-77.3	0.41 (8)	5.43	11-6	-234 / 459	0.30 (7)	
7-8	-1415 / 675	-77.3	-77.3	0.41 (8)	5.25	11- 7	-259 / 311	0.21 (4)	
15- 2	-1434 / 686	0.0	0.0	0.14(1)	6.86	10- 7	-257 / 194	0.09(3)	
9-8	-1301 / 619	0.0	0.0	0.13(1)	7.12	2-14	-457 / 1319	0.28(1)	
						10-8	-465 / 1239	0.27(1)	
15-14	-338 / 329	-17.5	-17.5	0.10 (11	6.25				
14-13	-653 / 1379	-17.5	-17.5	0.28 (1)	6.25				
13-12	-403 / 1175	-17.5	-17.5	0.25 (1)	6.25				
12-11	-403 / 1 <u>175</u>	-17.5	-17.5	0.25 (1)	6.25				
11-10	417 / 1			(	6.25				
10-9	/ 2	17	- 5	0.0 11	6.25				
							Б	EOEN /F	
WIND	APPELED						Y PRESSURE		
{40-0-0	)} FT-IN-SX REF	ERENCE H	HEIGHT	ABOVE	GRADE.	AND US	INC EXTERMA	AN REARKIVIII	TO

COE WIN {OPE FRO READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED

IN THE DESIGN OF THIS COMPONENT.

EXTERNAL REAR MILTON ING SYSTEM) INTERNAL MAY BIF INCRATED 2017 EAST (0-0) FT-IN-SX AWAY 17-4978

**BUILDING DIVISION** 

TOTAL WEIGHT = 2 X 122 = 244 **DESIGN CRITERIA** 

PSF

SPECIFIED LOADS: CH.

LL = 23.3 DL = 3.0 LL = 0.0 DL = 7.0 AD = 33.3 TOTAL LOAD

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09 - TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.90") CALCULATED VERT. DEFL.(LL)= L/ 999 (0.05") ALLOWABLE DEFL.(TL)= L/360 (0.90") CALCULATED VERT. DEFL.(TL)= L/ 999 (0.12")

CSI: TC=0.45 (3-4:7) , BC=0.28 (13-14:1) , WB=0.30 (6-11:7) , SSI=0.17 (2-3:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

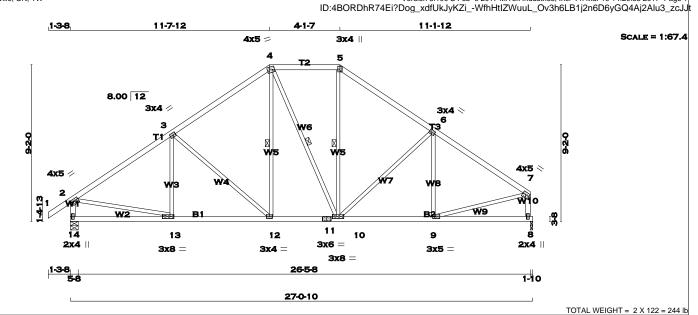
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (6) (INPUT = 0.90 ) JSI METAL= 0.52 (2) (INPUT = 1.00 )



JOB PESCENE PARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY DRWG NO. Page 65 of 159 TW0317-048 TW0317-048 T07 Kott Lumber Uxbridge, Stouffville, ON, TW Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:38 2017 Page 1



LUMBER	LUMBER									
N. L. G. A. R	N. L. G. A. RULES									
CHORDS	SIZE		LUMBER	DESCR.						
1 - 4	2x4	DRY	No.2	SPF						
4 - 5	2x4	DRY	No.2	SPF						
5 - 7	2x4	DRY	No.2	SPF						
14 - 2	2x4	DRY	No.2	SPF						
8 - 7	2x4	DRY	No.2	SPF						
14 - 11	2x4	DRY	No.2	SPF						
11 - 8	2x4	DRY	No.2	SPF						
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF						

DRY: SEASONED LUMBER.

PL/	PLATES (table is in inches)									
JT	TYPE	PLATES	W	LEN	Υ	Χ				
2	TMVW-t	MT20	4.0	5.0	1.50	2.00				
3	TMWW-t	MT20	3.0	4.0	1.50	1.50				
4	TTWW-m	MT20	4.0	5.0	1.75	1.50				
5	TTW+p	MT20	3.0	4.0	2.50	1.50				
6	TMWW-t	MT20	3.0	4.0	1.50	1.50				
7	TMVW-t	MT20	4.0	5.0	1.75	Edge				
8	BMV1+p	MT20	2.0	4.0		-				
9	BMWW-t	MT20	3.0	5.0	1.50	1.75				
10	BMWWW-t	MT20	3.0	8.0						
11	BS-t	MT20	3.0	6.0						
12	BMWW-t	MT20	3.0	4.0						
13	BMWW-t	MT20	3.0	8.0	1.50	2.75				
14	BMV1+n	MT20	2.0	4.0						

Edge - INDICATES REFERENCE CORNER OF PLATE

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS	AND LOADINGS	SPECIFIED B	Y FABRICATOR	TO BE VERIFIED E	3Y
BUILDING DESIGNER					
REARINGS					

3EA	KINGS						
	FACTOR	ED	MAXIMU	IAXIMUM FACTORED			REQRD
	<b>GROSS RE</b>	ACTION	GROSS REACTION			BRG	BRG
ΙT	VERT	HORZ	DOWN	HORZ	<b>UPLIFT</b>	IN-SX	IN-SX
4	1383	0	1473	398	-638	5-8	5-8
3	1276	0	1347	0	-571	1-10	1-10

PROVIDE ANCHORAGE AT BEARING JOINT 14 FOR 638 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 8 FOR 571 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES

PROVIDE FOR 398 LBS FACTORED HORIZONTAL REACTION AT JOINT 14

UNFACTORED REACT	IONS
1ST LCASE	MAX./

	1ST LCASE	MAX./N	<u>IIN. COMPO</u>	<u>NENT REACTION</u>	JNS		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
14	968	691 / 0	0/0	0/0	226 / -634	277 / 0	0/0
8	896	627 / 0	0/0	0/0	177 / -581	269 / 0	0/0
HOR 14	IZONTAL REA	ACTIONS 0/0	0/0	0/0	285 / -270	0/0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 14, 8

В

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.89 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 4-12, 4-10, 5-10. DBS = 20-0-0. CBF =

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING TOTAL LOAD CASES: (11)

СН	ORDS				WE	BS	
MA)	X. FACTORED	FACTORED				MAX. FACTO	ORED
MEMB.	FORCE	VERT. LOAD L	C1 MAX	MAX.	MEMB.	FORCE	MAX
	(LBS)	(PLF)	CSI (LC)	UNBRA	С	(LBS)	CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO	, ,	
1-2	0 / 29	-77.3 -77.	3 0.10 (1)	10.00	13- 3	-120 / 163	0.05(3)
2-3	-1563 / 698	-77.3 -77.	3 0.56 (7)	4.89	3-12	-476 / 443	0.56 (3)
3-4	-1252 / 687	-77.3 -77.	3 0.55 (7)	5.34	12-4	-232 / 391	0.12 (7)
4- 5	-1007 / 662	-77.3 -77.	3 0.25 (8)	6.09	4-10	-166 / 169	0.11 (5)
5-6	-1234 / 681	-77.3 -77.	3 0.52 (8)	5.44	10- 5	-174 / 345	0.09 (7)
6- 7	-1444 / 650	-77.3 -77.	3 0.51 (8)	5.09	10-6	-389 / 393	0.45 (4)
14-2	-1430 / 666	0.0 0.	0 0.14 (1)	6.87	9-6	-193 / 188	0.09 (3)
8- 7	-1306 / 598	0.0 0.	0 0.13(1)	7.11	2-13	-414 / 1318	0.29(1)
					9- 7	-426 / 1254	0.27(1)
14-13	-377 / 369	-17.5 -17.	5 0.15 (11	1) 6.25			
13-12	-638 / 1396	-17.5 -17.	5 0.28 (1)	6.25			
12-11	-306 / 1040	-17.5 -17.	5 0.20 (1)	6.25			
11-10	-306 / 1040	-17.5 -17.	5 0.20 (1)	6.25			
10-9	-386 / 1217	-17.5 -17.	5 0.25 (1)	6.25			
9-8	-1' / 2		(	) 6.25			
WIND	AD PP (L	R D	OM R E	RENCE V	ELOCIT	Y P <mark>RESSURE</mark>	PER POR PSE

(40-06) IN-S. T.E. RELICE HEIGHT ABOVE GRADE AND USING EXTERNAL CEDEN VED COEFFICIENTS, CPCg, BASED ON THE (MAIN WIND FORCE RESIS) INC. SYSTEM FIRMAL

WIN READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

INCTOVATEMONTEMINATION
MAY BE LOCATED ON EAST (MART-1209, X2011) AY

17-4978 **BUILDING DIVISION**  **DESIGN CRITERIA** 

SPECIFIED LOADS: LL = 23.3 DL = 3.0 LL = 0.0 DL = 7.0 AD = 33.3 TOP CH. PSF TOTAL LOAD

## SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014

- CSA 086-09 - TPIC 2011

> (55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.90") CALCULATED VERT. DEFL.(LL)= L/ 999 (0.05") ALLOWABLE DEFL.(TL)= L/360 (0.90") CALCULATED VERT. DEFL.(TL)= L/ 999 (0.10")

CSI: TC=0.56 (2-3:7) , BC=0.28 (12-13:1) , WB=0.56 (3-12:3) , SSI=0.19 (2-3:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

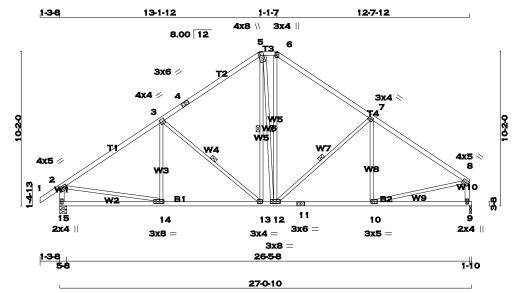
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (10) (INPUT = 0.90) JSI METAL= 0.52 (2) (INPUT = 1.00)



JOB PR-GREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY PLY DRWG NO. Page 66 of 159 2 TW0317-048 TW0317-048 T08

Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:38 2017 Page 1 Kott Lumber Uxbridge, Stouffville, ON, TW ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-WfhHtlZWuuL\_Ov3h6LB1j2n4l6ypQ83j2Alu3\_zcJJt



LUMBER				
N. L. G. A. R	RULES			
CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 5	2x4	DRY	No.2	SPF
5 - 6	2x4	DRY	No.2	SPF
6 - 8	2x4	DRY	No.2	SPF
15 - 2	2x4	DRY	No.2	SPF
9 - 8	2x4	DRY	No.2	SPF
15 - 11	2x4	DRY	No.2	SPF
11 - 9	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

PL/	PLATES (table is in inches)							
JT	TYPE	PLATES	W	LEN	Υ	Χ		
2	TMVW-t	MT20	4.0	5.0	1.50	2.00		
3	TMWW-t	MT20	4.0	4.0	2.00	1.50		
4	TS-t	MT20	3.0	6.0				
5	TTWW+m	MT20	4.0	8.0	2.50	1.00		
6	TTW+p	MT20	3.0	4.0	2.50	1.50		
7	TMWW-t	MT20	3.0	4.0	1.50	1.50		
8	TMVW-t	MT20	4.0	5.0	1.75	Edge		
9	BMV1+p	MT20	2.0	4.0				
10	BMWW-t	MT20	3.0	5.0	1.50	1.75		
11	BS-t	MT20	3.0	6.0				
12	BMWWW-t	MT20	3.0	8.0				
13	BMWW-t	MT20	3.0	4.0				
14	BMWW-t	MT20	3.0	8.0	1.50	3.25		
15	RMV/1+n	MT20	2.0	4 0				

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

14. L. O. / t. 10	OLLO				
CHORDS	SIZE		LUMBER	DESCR.	
1 - 4	2x4	DRY	No.2	SPF	
4 - 5	2x4	DRY	No.2	SPF	
5 - 6	2x4	DRY	No.2	SPF	
6 - 8	2x4	DRY	No.2	SPF	
15 - 2	2x4	DRY	No.2	SPF	
9 - 8	2x4	DRY	No.2	SPF	
15 - 11	2x4	DRY	No.2	SPF	
11 - 9	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	
DRY: SEASO	ONED L	UMBER.			

DIMENSIONS, SUPPORTS	AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY
BUILDING DESIGNER	
BEARINGS	

BEA	RINGS						
	FACTO	RED	MAXIMU	M FACT	ORED	INPUT	REQRD
	GROSS R	EACTION	GROSS REACTION			BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
15	1383	0	1471	438	-611	5-8	5-8
9	1276	0	1354	0	-544	1-10	1-10

PROVIDE ANCHORAGE AT BEARING JOINT 15 FOR 611 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 9 FOR 544 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES

PROVIDE FOR 438 LBS FACTORED HORIZONTAL REACTION AT JOINT 15

	1ST LCASE	MAX./N	IIN. COMPON	NENT REACTION	ONS		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	S
15	968	691 / 0	0/0	0/0	222 / -615	277 / 0	0
9	896	627 / 0	0/0	0/0	193 / -562	269 / 0	C
HOR 15	IZONTAL REA	ACTIONS 0/0	0/0	0/0	313 / -298	0/0	O
BEAR	RING MATER	IAL TO BE SI	PF NO.2 OR I	BETTER AT JO	DINT(S) 15, 9		
TOP MAX				PURLIN SPACI TH = 6.25 FT.			TLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 3-13, 5-13, 5-12, 6-12, 7-12. DBS =

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING TOTAL LOAD CASES: (11)

UNFACTORED REACTIONS

	. 5 / 12 25 / 15 5 / 16 25 ( · · · )								
	ORDS					WE			
MA2	X. FACTORED	FACTO	RED				MAX. FACTO	RED	
MEMB.	FORCE	VERT. LC	AD LC	1 MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PL	_F) (	CSI (LC)	<b>UNBRAC</b>	2	(LBS)	CSI (LC)	
FR-TO		FROM	TO		LENGTH	FR-TO			
1-2	0 / 29	-77.3	-77.3	0.10(1)	10.00	14-3	-78 / 161	0.04 (4)	
2-3	-1554 / 656	-77.3	-77.3	0.68 (7)	4.72	3-13	-594 / 528	0.29 (3)	
3- 4	-1156 / 646	-77.3	-77.3	0.67 (7)	5.35	13- 5	-255 / 397	0.18 (7)	
4- 5	-1156 / 646	-77.3	-77.3	0.67 (7)	5.35	5-12	-170 / 164	0.12 (5)	
5-6	-941 / 620	-77.3	-77.3	0.10 (8)	6.25	12-6	-246 / 404	0.17 (7)	
6- 7	-1162 / 655	-77.3	-77.3	0.63 (8)	5.43	12-7	-503 / 478	0.24 (4)	
7-8	-1456 / 615	-77.3	-77.3	0.63 (8)	4.91	10- 7	-150 / 185	0.08 (3)	
15-2	-1424 / 644	0.0	0.0	0.14(1)	6.88	2-14	-393 / 1325	0.31 (8)	
9-8	-1307 / 575	0.0	0.0	0.13(1)	7.11	10-8	-399 / 1261	0.30(7)	
15-14	-416 / 408	-17.5	-17.5	0.19 (11)	6.25				
14-13	-617 / 1401	-17.5	-17.5	0.30(1)	6.25				
13-12	-215 / 951	-17.5	-17.5	0.22(1)	6.25				
12-11	-362/1231	<u>-17.</u> 5	-17.5	0.27 (1)	6.25				
11-10	ડે <sup>ફ</sup> / 1:			(	6.25				
10-9	/ 2	17	- 5	0.1 11	6.25				
ſ							D	CCEN/E	$\overline{}$
WIND	APPELED	VE	D 👯 O	M REFER	RENCE V	ELOCIT	Y PRESSURE		FLA
{40-0-0	)} FT-IN-SX REF	ERENCE I	HEIGH1	ABOVE	GRADE	AND US	INC EXTERNAL	<b>NPOF</b> MIL	$_{TC}$

COE WIN {OPE FRO READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

EXTERMAL REAK MILTON ING SYSTEM).INTERNAL MAY BIMAR ATED 2017 LEAST (0-0) FT-IN-SX AWAY 17-4978

SOIL

0/0

0 /0

**BUILDING DIVISION** 

TOTAL WEIGHT = 2 X 128 = 256 lb

SCALE = 1:75.7

# **DESIGN CRITERIA** SPECIFIED LOADS:

LL = 23.3 DL = 3.0 LL = 0.0 DL = 7.0 AD = 33.3 TOP CH. PSF TOTAL LOAD

## SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09

- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.90")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.90")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.12")

CSI: TC=0.68 (2-3:7) , BC=0.30 (13-14:1) , WB=0.31 (2-14:8) , SSI=0.22 (2-3:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

# NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (10) (INPUT = 0.90) JSI METAL= 0.53 (2) (INPUT = 1.00)



JOB PESC TR-GREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY PLY DRWG NO. TRUSS DESC TW0317-048 T09 3

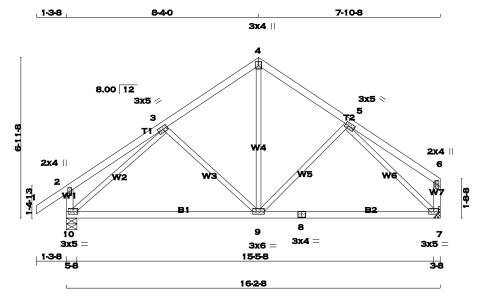
Kott Lumber Uxbridge, Stouffville, ON, TW

Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:39 2017 Page 1 ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-\_sFf4ea8fCTr03eug3iGFGKLKWHy9YrsHqURbRzcJJs

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SCALE = 1:49.9

TW0317-048



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR 2x4 DRY No.2 No.2 SPF SPF 2x4 10 -7 -10 -2x4 DRY No 2 SPF 2x4 No.2 8 2x4 DRY No.2 SPF DRY No.2 ALL WERS 2x3 DRY No.2 SPF EXCEPT

DRY: SEASONED LUMBER

PLATES (table is in inches)								
JT	TYPE	PLATES	W	LEN	Υ	X		
	TMV+p	MT20	2.0	4.0				
3	TMWW-t	MT20	3.0	5.0				
4	TTW+p	MT20	3.0	4.0	2.25	1.50		
5	TMWW-t	MT20	3.0	5.0	1.50	2.00		
6	TMV+p	MT20	2.0	4.0				
7	BMVW1-t	MT20	3.0	5.0				
8	BS-t	MT20	3.0	4.0				
9	BMWWW-t	MT20	3.0	6.0				
10	BMVW1-t	MT20	3.0	5.0				

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

# DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEA	BEARINGS									
	FACTOR	RED	MAXIMUI	M FACTO	INPUT	REQRD				
	<b>GROSS RE</b>	ACTION	GROSS REACTION			BRG	BRG			
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX			
10	875	0	928	311	-382	5-8	5-8			
7	769	0	813	0	-317	HANGER E	BY OTHERS			
						MIN. SEAT	SIZE: 3-8			

PROVIDE ANCHORAGE AT BEARING JOINT 10 FOR 382 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 7 FOR 317 LBS FACTORED UPLIFT UPLIFT

PROVIDE FOR 311 LBS FACTORED HORIZONTAL REACTION AT JOINT 10

## UNFACTORED REACTIONS

	1ST LCASE	MAX.	MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
10	612	441 / 0	0/0	0/0	132 / -382	170 / 0	0/0		
7	539	377 / 0	0/0	0/0	112 / -331	162 / 0	0/0		
HOR 10	IZONTAL RE	ACTIONS 0/0	0/0	0/0	222 / -208	0/0	0 /0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 10

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (11)

CHC	RDS				WEBS				
MAX.	FACTORED	FACTOR	RED				MAX. FACT	ORED	
MEMB.	FORCE	VERT. LO	AD LC1	I MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PL	F) (	CSI (LC)	<b>UNBRAC</b>	2	(LBS)	CSI (LC)	
FR-TO		FROM	TO		LENGTH	FR-TO			
1- 2	0 / 29	-77.3	-77.3	0.10(1)	10.00	3- 9	-262 / 372	0.14 (3)	
2-3	-26 / 170	-77.3	-77.3	0.25 (7)	6.25	9- 4	-239 / 443	0.19 (7)	
3- 4	-658 / 363	-77.3	-77.3	0.28 (7)	6.25	9- 5	-200 / 327	0.10 (4)	
4- 5	-658 / 371	-77.3	-77.3	0.25 (8)	6.25	10- 3	-922 / 292	0.47 (4)	
5-6	-24 / 115	-77.3	-77.3	0.22 (8)	6.25	5- 7	-890 / 312	0.44 (3)	
10- 2	-267 / 257	0.0	0.0	0.04 (7)	7.81				
7-6	-147 / 156	0.0	0.0	0.03 (8)	7.81				
10- 9	-351 / 727	-17.5	-17.5	0.37 (11	6.25				
9-8	-191 / 623			0.37 (11					
8- 7	-191 / 623			0.37 (11					

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT WIND LOAD APPLIED IS DERIVED FROM REPERSING VELOCITY FRESTURE OF (\$3.0) FOR AL (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.





READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

**RECEIVED** TOWN OF MILTON MAR 29, 2017 17-4978 **BUILDING DIVISION** 

# **DESIGN CRITERIA**

SPECIFIED LOADS:								
TOP	CH.	LL	=	23.3	PSF			
		DL	=	3.0	PSF			
BOT	CH.	LL	=	0.0	PSF			
		DL	=	7.0	PSF			
TOTA	L LO	AD	=	33.3	PSF			

## SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

TOTAL WEIGHT = 3 X 68 = 203 lb

THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012, BCBC 2012, ABC 2014 - CSA 086-09

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.54") CALCULATED VERT. DEFL.(LL) = L/999 (0.02") ALLOWABLE DEFL.(TL)= L/360 (0.54") CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.28 (3-4:7) , BC=0.37 (9-10:11) , WB=0.47 (3-10:4) , SSI=0.14 (3-4:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

TPIC 2011

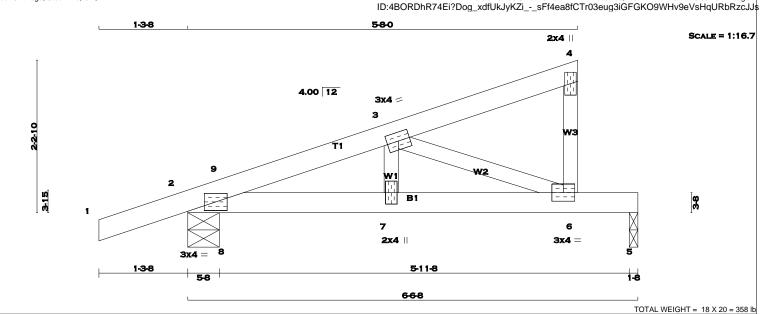
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (3) (INPUT = 0.90 ) JSI METAL= 0.26 (3) (INPUT = 1.00 )

POBPES GREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY PLY DRWG NO. Page 68 of 159 TW0317-048 TW0317-048 T10 18 Kott Lumber Uxbridge, Stouffville, ON, TW Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:39 2017 Page 1



LUMBER								
N. L. G. A. RULES								
CHORDS	SIZE		LUMBER	DESCR.				
1 - 4	2x4	DRY	No.2	SPF				
6 - 4	2x3	DRY	No.2	SPF				
2 - 5	2x4	DRY	No.2	SPF				
ALL WEBS	2x3	DRY	No.2	SPF				
DRY: SEASO	ONED L	UMBER						

PLATES (table is in inches)									
JT	TYPE	PLATES	W	LEN Y	X				
2	TMB1-I	MT20	3.0	4.0					
3	TMWW-t	MT20	3.0	4.0					
4	TMV+p	MT20	2.0	4.0					
6	BMVW-t	MT20	3.0	4.0					
7	BMW+w	MT20	2.0	4.0					

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

# DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEAR	BEARINGS										
	FACTORED		MAXIMUM FACTORED			INPUT	REQRD				
	<b>GROSS RE</b>	ACTION	GROSS REACTION			BRG	BRG				
JT	VERT	HORZ	DOWN	HORZ	<b>UPLIFT</b>	IN-SX	IN-SX				
2	409	0	409	151	-217	5-8	5-8				
5	247	0	247	0	-121	1-8	1-8				

PROVIDE ANCHORAGE AT BEARING JOINT 2 FOR 217 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 5 FOR 150 LBS FACTORED UPLIFT

PROVIDE FOR 151 LBS FACTORED HORIZONTAL REACTION AT JOINT 2

UNF	UNFACTORED REACTIONS										
	1ST LCASE	MAX./	MAX./MIN. COMPONENT REACTIONS								
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL				
2	285	212/0	0/0	0/0	0 / -202	73 / 0	0/0				
5	175	114/0	0/0	0/0	0 / -125	61 / 0	0/0				
HOF	HORIZONTAL REACTIONS										
2		0/0	0/0	0/0	108 / 0	0/0	0 /0				

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 2, 5

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (11)

СНС	RDS					WE	BS	
MAX.	FACTORED	FACTO	RED				MAX. FACTO	ORED
MEMB.	FORCE	VERT. LO	AD LC	1 MAX	MAX.	MEMB.	FORCE	MAX
	(LBS)	(PL	.F) (	CSI (LC)	<b>UNBRAC</b>		(LBS)	CSI (LC)
FR-TO		FROM	TO		LENGTH	FR-TO		
1- 2	0 / 15	-77.3	-77.3	0.10(1)	10.00	7-3	-30 / 178	0.04 (1)
2- 9	-607 / 273	-77.3	-77.3	0.04 (5)	6.25	3-6	-606 / 361	0.11 (1)
9- 3	-593 / 303	-77.3	-77.3	0.09(5)	6.25	8- 9	-74 / 84	0.00(1)
3- 4	-43 / 23	-77.3	-77.3	0.06(1)	6.25			
6- 4	-91 / 67	0.0	0.0	0.06 (5)	7.81			
2-8	-300 / 568	-17.5	-17.5	0.14(1)	6.25			
8- 7	-300 / 568	-17.5	-17.5	0.16(1)	6.25			
7-6	-300 / 568	-17.5	-17.5	0.38 (1)	6.25			
6- 5	0/0	-17.5	-17.5	0.29(1)	10.00			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT { 40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

## **DESIGN CRITERIA**

SPECIFIED LOADS: CH. PSF

LL = 23.3 DL = 3.0 LL = 0.0 DL = 7.0 AD = 33.3 PSF TOTAL LOAD PSF

## SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012, BCBC 2012, ABC 2014 - CSA 086-09 TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.22")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.22") CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.10 (1-2:1), BC=0.38 (6-7:1), WB=0.11 (3-6:1) , SSI=0.19 (5-6:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (6) (INPUT = 0.90 ) JSI METAL= 0.21 (6) (INPUT = 1.00 )

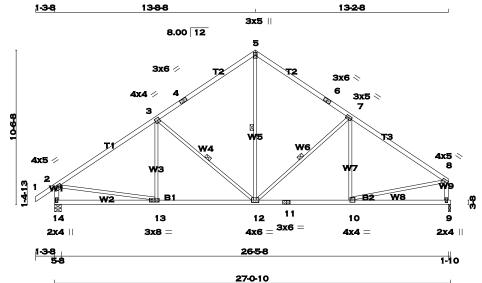




READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

POBPES GREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY DRWG NO. Page 69 of 159 TRUSS DESC. T11 TW0317-048 TW0317-048 2 Kott Lumber Uxbridge, Stouffville, ON, TW

Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:39 2017 Page ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-\_sFf4ea8fCTr03eug3iGFGKEIWI19arsHqURbRzcJJs



LUMBER				
N. L. G. A. F	RULES			
CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 5	2x4	DRY	No.2	SPF
5 - 6	2x4	DRY	No.2	SPF
6 - 8	2x4	DRY	No.2	SPF
14 - 2	2x4	DRY	No.2	SPF
9 - 8	2x4	DRY	No.2	SPF
14 - 11	2x4	DRY	No.2	SPF
11 - 9	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PL/	PLATES (table is in inches)								
JT	TYPE	PLATES	W	LEN	Υ	X			
2	TMVW-t	MT20	4.0	5.0	1.50	2.00			
3	TMWW-t	MT20	4.0	4.0	2.00	1.50			
4	TS-t	MT20	3.0	6.0					
5	TTW+p	MT20	3.0	5.0					
6	TS-t	MT20	3.0	6.0					
7	TMWW-t	MT20	3.0	5.0					
8	TMVW-t	MT20	4.0	5.0	1.75	Edge			
9	BMV1+p	MT20	2.0	4.0					
10	BMWW-t	MT20	4.0	4.0	1.75	1.50			
11	BS-t	MT20	3.0	6.0					
12	BMWWW-t	MT20	4.0	6.0					
13	BMWW-t	MT20	3.0	8.0	1.50	3.50			
14	BMV1+p	MT20	2.0	4.0					

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

## DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

3EA	EARINGS										
	FACTOR	RED	MAXIMUM FACTORED			INPUT	REQRD				
	GROSS RE	ACTION	GROSS REACTION			BRG	BRG				
JΤ	VERT	HORZ	DOWN	HORZ	<b>UPLIFT</b>	IN-SX	IN-SX				
14	1383	0	1470	452	-601	5-8	5-8				
9	1276	0	1356	0	-533	1-10	1-10				

PROVIDE ANCHORAGE AT BEARING JOINT 14 FOR 601 LBS\_FACTORED PROVIDE ANCHORAGE AT BEARING JOINT 9 FOR 533 LBS\_FACTORED

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES

PROVIDE FOR 452 LBS FACTORED HORIZONTAL REACTION AT JOINT 14

UNFACTORED REACT	TIONS
10710105	

	1ST LCASE	MAX./	MIN. COMPON	<u>IENT REACTIO</u>	JNS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL			
14	968	691 / 0	0/0	0/0	219 / -607	277 / 0	0/0			
9	896	627 / 0	0/0	0/0	199 / -554	269 / 0	0/0			
HOR	HORIZONTAL REACTIONS									
14		0/0	0/0	0/0	323 / -309	0/0	0 /0			

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 14, 9

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 3-12, 5-12, 7-12. DBS = 20-0-0 . CBF =

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING TOTAL LOAD CASES: (11)

		,							
	ORDS					WE			
MAX	(. FACTORED	FACTOR	RED				MAX. FACTO	DRED	
MEMB.	FORCE	VERT. LO.	AD LC1	I MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PL	F) (	CSI (LC)	UNBRAC	2	(LBS)	CSI (LC)	
FR-TO		FROM	TO		LENGTH	FR-TO			
1- 2	0 / 29	-77.3	-77.3	0.10(1)	10.00	13- 3	-69 / 160	0.04 (4)	
2-3	-1545 / 637	-77.3	-77.3	0.73(7)	4.66	3-12	-626 / 554	0.33(3)	
	-1135 / 638						-442 / 800	0.33 (7)	
4- 5	-1135 / 638	-77.3	-77.3	0.72 (7)	5.32	12- 7	-543 / 509	0.28 (4)	
5-6	-1135 / 646	-77.3	-77.3	0.67 (8)	5.40	10- 7	-132 / 178	0.08(3)	
6- 7	-1135 / 646	-77.3	-77.3	0.67 (8)	5.40	2-13	-385 / 1326	0.34 (8)	
7-8	-1458 / 600	-77.3	-77.3	0.68 (8)	4.84	10-8	-394 / 1262	0.33 (7)	
14- 2	-1420 / 634	0.0	0.0	0.14(1)	6.89				
9-8	-1308 / 566	0.0	0.0	0.13(1)	7.11				
14-13	-431 / 422	-17.5	-17.5	0.20 (11	) 6.25				
13-12	-605 / 1399	-17.5	-17.5	0.30(1)	6.25				
12-11	-358 / 1235	-17.5	-17.5	0.29(1)	6.25				
11-10_	<u>-3</u> 58 / 1235	<u>-17.</u> 5	-17.5	0.29(1)	6.25				
10-9	-1' /2			(	) 6.25				
WIND	D AD PP £L						Y PRESSURE		F
{40-0	;──-IN-SXTE	LRE F	IE.UHT	ABOVE	GRADE.	AND US	INC EXTERNA		ΞL

COEFFICIENTS, CPCG, BASED ON THE (MAIN WIND FORCE RESIS INCE) WIND FORCE RESIS INCE) WIND FORCE RESIS INCE ON THE MAY BE LOCATED ON THE COPE READ ALL NOTES ON THIS PAGE AND ON THE LEAST (MART 105) 2007 ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED

IN THE DESIGN OF THIS COMPONENT.

17-4978 **BUILDING DIVISION**  **DESIGN CRITERIA** 

SPECIFIED LOADS: LL = DL = LL = DL = AD = CH. 3.0 PSF 7.0 PSF

## SPACING = 24.0 IN. C/C

TOTAL LOAD

- TPIC 2011

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

33.3

TOTAL WEIGHT = 2 X 114 = 228 II

SCALE = 1:78.7

THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.90") CALCULATED VERT. DEFL.(LL) = L/999 (0.06") ALLOWABLE DEFL.(TL)= L/360 (0.90") CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.73 (2-3:7) , BC=0.30 (12-13:1) , WB=0.34 (2-13:8) , SSI=0.23 (2-3:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

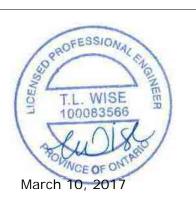
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (8) (INPUT = 0.90 ) JSI METAL= 0.53 (2) (INPUT = 1.00 )



JOB PR-GREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY PLY DRWG NO. Page 70 of 159 5 TW0317-048 TW0317-048 T12 Kott Lumber Uxbridge, Stouffville, ON, TW Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:40 2017 Page 1

> 1-3-8 SCALE = 1:16.4 2x4 # 4.00 12 3x4 = WЗ Ŵı В1 8 5 3x4 = 1-3-8 5-8 4118

ULES			
SIZE		LUMBER	DESCR.
2x4	DRY	No.2	SPF
2x3	DRY	No.2	SPF
2x4	DRY	No.2	SPF
2x4	DRY	No.2	SPF
2x3	DRY	No.2	SPF
NED L	JMBER.		
	2x4 2x3 2x4 2x4 2x4	SIZE 2x4 DRY 2x3 DRY 2x4 DRY 2x4 DRY 2x4 DRY	SIZE         LUMBER           2x4         DRY         No.2           2x3         DRY         No.2           2x4         DRY         No.2           2x4         DRY         No.2           2x3         DRY         No.2

## PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Υ	Χ	
2	TMVW-t	MT20	3.0	4.0	1.50	1.50	
3	TMV+p	MT20	2.0	4.0			
5	BMVW-t	MT20	3.0	4.0			
6	BMV1+p	MT20	2.0	4.0			

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

## DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

DEA	BEARINGS											
	FACTORED		MAXIMUM FACTORED			INPUT	REQRD					
	GROSS R	EACTION	GROSS	GROSS REACTION			BRG					
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX					
6	333	0	333	155	-174	5-8	5-8					
4	173	0	173	0	-87	1-8	1-8					

PROVIDE ANCHORAGE AT BEARING JOINT 6 FOR 174 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 4 FOR 150 LBS FACTORED UPLIFT

PROVIDE FOR 155 LBS FACTORED HORIZONTAL REACTION AT JOINT 6

## UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS									
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
6	231	174 / 0	0/0	0/0	0 / -161	57 / 0	0/0		
4	123	78 / 0	0/0	0/0	0 / -91	45 / 0	0/0		
HORIZONTAL REACTIONS									
6		0/0	0/0	0/0	111 / 0	0/0	0 /0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 6, 4

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (11)

CHO	CHORDS				WEBS				
MAX.	FACTORED	FACTOR	RED				MAX. FACTO	RED	
MEMB.	FORCE	VERT. LO	AD LC1	1 MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PL	F) (	CSI (LC)	UNBRAC		(LBS)	CSI (LC)	
FR-TO		FROM	TO		LENGTH	FR-TO			
1- 2	0 / 16	-77.3	-77.3	0.10(1)	10.00	2- 5	0 / 120	0.02 (6)	
2-3	-59 / 11	-77.3	-77.3	0.22 (1)	6.25				
5-3	-158 / 130	0.0	0.0	0.08 (5)	7.81				
6- 2	-261 / 181	0.0	0.0	0.03 (7)	7.81				
	-141 / 0	-17.5	-17.5	0.20(1)	6.25				
5- 4	0/0	-17.5	-17.5	0.20(1)	10.00				

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.

# **DESIGN CRITERIA**

ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-S2p1I\_bmPWbieDD4EmDVoTtX\_vfxu660VUE?7tzcJJ

SPECIFIED LOADS:								
TOP	CH.	LL	=	23.3	PSF			
		DL	=	3.0	PSF			
BOT	CH.	LL	=	0.0	PSF			
		DL	=	7.0	PSF			
TOTA	L LO	AD	=	33.3	PSF			

## SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

TOTAL WEIGHT = 5 X 17 = 85 lb

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014 - CSA 086-09 TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19") CALCULATED VERT. DEFL.(LL) = L/999 (0.03") ALLOWABLE DEFL.(TL)= L/360 (0.19") CALCULATED VERT. DEFL.(TL) = L/872 (0.07")

CSI: TC=0.22 (2-3:1) . BC=0.20 (5-6:1) . WB=0.02 (2-5:6) , SSI=0.14 (4-5:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.26 (2) (INPUT = 0.90 ) JSI METAL= 0.07 (6) (INPUT = 1.00 )





READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

JOB NAME

Kott Lumber Uxbridge, Stouffville, ON, TW

TW0317-048

TRUSS NAME

T13

QUANTITY PLY 2

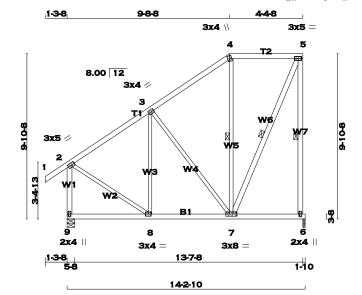
POBPES GREENPARK-LECCO RIDGE-BLOCK 327 TRUSS DESC

DRWG NO.

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SCALE = 1:68.8

Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:40 2017 Page 1 ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-S2p1I\_bmPWbieDD4EmDVoTtR3vh1u?30VUE?7tzcJJI



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR 4 2x4 DRY No.2 No.2 SPF SPF 2x4 DRY 6 5 2x4 DRY No 2 SPF No.2 2x4 DRY No.2 SPF ALL WEBS DRY No.2 SPF 2x3 EXCEPT 7 - 5

DRY: SEASONED LUMBER

PL/	ATES (table	is in inches	)				
JT	TYPE	PLATES	W	LEN	Υ	X	
2	TMVW-t	MT20	3.0	5.0	1.50	2.00	
3	TMWW-t	MT20	3.0	4.0	1.50	1.50	
4	TTW+m	MT20	3.0	4.0			
5	TMVW-t	MT20	3.0	5.0			
6	BMV1+p	MT20	2.0	4.0			
7	BMWWW-t	MT20	3.0	8.0			
8	BMWW-t	MT20	3.0	4.0			
9	BMV1+p	MT20	2.0	4.0			

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

3EAI	EARINGS										
	FACTOR	ED	MAXIMUM FACTORED			INPUT	REQRD				
	<b>GROSS RE</b>	ACTION	GROSS F	REACTIO	BRG	BRG					
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX				
3	668	0	752	0	-447	1-10	1-10				
9	774	0	814	555	-322	5-8	5-8				

PROVIDE ANCHORAGE AT BEARING JOINT 6 FOR 447 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 9 FOR 322 LBS FACTORED UPLIFT

PROVIDE FOR 555 LBS FACTORED HORIZONTAL REACTION AT JOINT 9

UNFACTORED REACTIONS

	1ST LCASE	MAX.	/MIN. COMPON	ENT REACTION						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL			
6	469	328 / 0	0/0	0/0	211 / -410	141 / 0	0/0			
9	541	392 / 0	0/0	0/0	100 / -326	149 / 0	0/0			
HORIZONTAL REACTIONS										
9		0/0	0/0	0/0	396 / -287	0/0	0 /0			

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 6, 9

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 5-6, 4-7, 5-7, DBS = 20-0-0, CBF = 80

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING TOTAL LOAD CASES: (11)

CHORDS				WEBS					
MAX.	FACTORED	FACTORED			MAX. FACTORED				
MEMB.	FORCE	VERT. LO	AD LC	1 MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PL	F) (	CSI (LC)	<b>UNBRAC</b>	2	(LBS)	CSI (LC)	
FR-TO		FROM	TO		LENGTH	FR-TO			
1- 2	0 / 29	-77.3	-77.3	0.10(1)	10.00	8- 3	-164 / 109	0.12 (4)	
2- 3	-470 / 267	-77.3	-77.3	0.36 (7)	6.25	3- 7	-357 / 398	0.47 (3)	
3- 4	-338 / 305	-77.3	-77.3	0.36 (7)	6.25	7- 4	-176 / 139	0.11 (3)	
4- 5	-256 / 335	-77.3	-77.3	0.22 (7)	6.25	7- 5	-363 / 596	0.20 (7)	
6- 5	-722 / 467	0.0	0.0	0.60 (7)	6.25	2-8	-75 / 494	0.10(1)	
9- 2	-779 / 346	0.0	0.0	0.14(1)	7.81				
9-8	-494 / 374	-17.5	-17.5	0.10 (11)	6.25				
8- 7	-375 / 457	-17.5	-17.5	0.13 (11)	6.25				
7- 6	-73 / 188	-17.5	-17.5	0.08 (11)	6.25				

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CPCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

**RECEIVED** TOWN OF MILTON MAR 29, 2017 17-4978 **BUILDING DIVISION**  **DESIGN CRITERIA** 

TOTAL WEIGHT = 2 X 85 = 170 lb

SPECIFIED LOADS:

LL = DL = LL = DL = AD = CH. PSF 3.0 PSF 7.0 PSF TOTAL LOAD 33.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09 - TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.47\*) CALCULATED VERT. DEFL.(LL)= L/ 999 (0.01\*) ALLOWABLE DEFL.(TL)= L/360 (0.47\*) CALCULATED VERT. DEFL.(TL)= L/999 (0.02\*)

CSI: TC=0.60 (5-6:7), BC=0.13 (7-8:11), WB=0.47 (3-7:3), SSI=0.16 (2-3:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (4) (INPUT = 0.90) JSI METAL= 0.19 (2) (INPUT = 1.00)



JOB NAME TRUSS NAME

TW0317-048

QUANTITY PLY 2

POBPES GREENPARK-LECCO RIDGE-BLOCK 327 TRUSS DESC

DRWG NO.

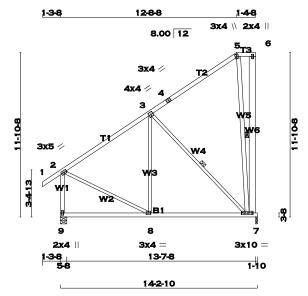
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SCALE = 1:83.1

Kott Lumber Uxbridge, Stouffville, ON, TW

T14

Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:40 2017 Page 1 ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-S2p1I\_bmPWbieDD4EmDVoTtRKveau?S0VUE?7tzcJJI



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR 4 2x4 DRY No.2 No.2 SPF SPF DRY 2x4 5 7 6 2x4 DRY No 2 SPF 2x6 No.2 2 2x4 DRY No.2 SPF No.2 SPF DRY ALL WERS 2x3 DRY No.2 SPF **EXCEPT** DRY SPF 2x4 No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Υ	X					
2	TMVW-t	MT20	3.0	5.0	1.50	2.00					
3	TMWW-t	MT20	4.0	4.0	2.00	1.50					
4	TS-t	MT20	3.0	4.0							
5	TTW+m	MT20	3.0	4.0	2.00	1.25					
6	TMV+p	MT20	2.0	4.0							
7	BMVWW1-t	MT20	3.0	10.0							
8	BMWW-t	MT20	3.0	4.0							
9	BMV1+p	MT20	2.0	4.0							

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

3EA	RINGS						
	FACTOR	ED	MAXIMUN	/ FACTO	INPUT	REQRD	
	<b>GROSS RE</b>	ACTION	GROSS F	REACTIO	N	BRG	BRG
ΙT	VERT	HORZ	DOWN	HORZ	<b>UPLIFT</b>	IN-SX	IN-SX
7	668	0	782	0	-490	1-10	1-10
9	774	0	836	667	-279	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 7 FOR 490 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 9 FOR 279 LBS FACTORED UPLIFT

PROVIDE FOR 667 LBS FACTORED HORIZONTAL REACTION AT JOINT 9

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS								
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
7	469	328 / 0	0/0	0/0	286 / -441	141 / 0	0/0	
9	541	392 / 0	0/0	0/0	156 / -295	149 / 0	0/0	
HORIZONTAL REACTIONS 9 0/0 0/0 0/0 477/-336 0/0 0/0							0 /0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 7, 9

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 6-7, 3-7, 5-7, DBS = 20-0-0, CBF = 66

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING TOTAL LOAD CASES: (11)

CHC	DRDS				WEBS					
MAX.	FACTORED	FACTO	RED				MAX. FACTO	DRED		
MEMB.	FORCE	VERT. LO	AD LC1	I MAX	MAX.	MEMB.	FORCE	MAX		
	(LBS)	(PL	.F) (	CSI (LC)	<b>UNBRAC</b>		(LBS)	CSI (LC)		
FR-TO		FROM	TO		LENGTH	FR-TO				
1- 2	0/29	-77.3	-77.3	0.10(1)	10.00	8- 3	-62 / 163	0.07 (4)		
2- 3	-501 / 206	-77.3	-77.3	0.58 (7)	6.25	3- 7	-611 / 553	0.45 (3)		
3- 4	-165 / 230	-77.3	-77.3	0.57 (7)	6.25	2-8	-120 / 501	0.11 (5)		
4- 5	-165 / 230	-77.3	-77.3	0.57 (7)	6.25	5- 7	-250 / 196	0.19 (3)		
5- 6	-89 / 227	-77.3	-77.3	0.05 (7)	6.25					
7- 6	-56 / 41	0.0	0.0	0.39 (7)	6.25					
9- 2	-796 / 307	0.0	0.0	0.14 (4)	7.81					
9-8	-607 / 442	-17.5	-17.5	0.26 (11	) 6.25					
8- 7	-373 / 464	-17.5	-17.5	0.28 (11	) 6.25					

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT (40-0-0) F-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK
COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL
WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON
(OPEN TERRAIN). AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

**RECEIVED** TOWN OF MILTON MAR 29, 2017 17-4978 **BUILDING DIVISION**  **DESIGN CRITERIA** 

TOTAL LOAD

SPECIFIED LOADS: LL = DL = LL = DL = AD = CH. PSF 3.0 PSF 7.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

TOTAL WEIGHT = 2 X 93 = 186 lb

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

33.3

THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09 - TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.47\*) CALCULATED VERT. DEFL.(LL)= L/ 999 (0.01\*) ALLOWABLE DEFL.(TL)= L/360 (0.47\*) CALCULATED VERT. DEFL.(TL)= L/999 (0.08\*)

CSI: TC=0.58 (2-3:7), BC=0.28 (7-8:11), WB=0.45 (3-7:3), SSI=0.21 (2-3:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

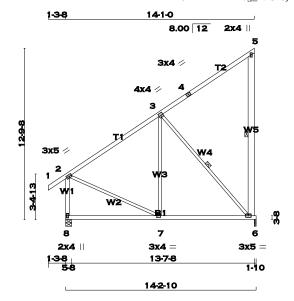
JSI GRIP= 0.67 (2) (INPUT = 0.90) JSI METAL= 0.21 (2) (INPUT = 1.00)



POBPES GREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY PLY DRWG NO. Page 73 of 159 TRUSS DESC T15 2 TW0317-048 TW0317-048

Kott Lumber Uxbridge, Stouffville, ON, TW

Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:41 2017 Page ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-wEMPVKcPApjZFNoGnTkkKhPbBJzwdRc9k8zYgJzcJJq



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR 2x4 DRY No.2 No.2 SPF SPF 6 2x6 DRY No 2 SPF No.2 8 6 2x4 DRY No.2 SPF ALL WEBS 2x3 DRY No.2 SPF EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)
JT TYPE PLATES
2 TMVW-t MT20 W LEN Y 3.0 1.50 2.00 5.0 4.0 4.0 TMWW-1 MT20 4.0 2.00 1.50 TMV+p MT20 2.0 4.0 BMVW1-t MT20 3.0 5.0 1.50 2.25 BMWW-t MT20 4.0 BMV1+p 20 40

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS FACTORED MAXIMUM FACTORED INPLIT REQRD GROSS REACTION GROSS REACTION HORZ UPLIFT IN-SX .IT VFRT HOR7 DOWN IN-SX -512 1-10 766 -257 5-8 5-8

PROVIDE ANCHORAGE AT BEARING JOINT 6 FOR 512 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES

PROVIDE FOR 766 LBS FACTORED HORIZONTAL REACTION AT JOINT 8

UNFACTORED REACTIONS

	1ST LCASE	ASE MAX./MIN. COMPONENT REACTIONS								
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL			
6	469	328 / 0	0/0	0/0	336 / -456	141 / 0	0/0			
8	541	392 / 0	0/0	0/0	202 / -279	149 / 0	0/0			
HORIZONTAL REACTIONS										
8		0/0	0/0	0/0	547 / -381	0/0	0 /0			

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 6, 8

8

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 5-6, 3-6. DBS = 20-0-0 . CBF = 75 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING TOTAL LOAD CASES: (11)

СН	ORDS					WE	BS		
MA)	K. FACTORED	FACTO	RED				MAX. FACTO	DRED	
MEMB.	FORCE	VERT. LO	AD LC	1 MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PL	.F) (	CSI (LC)	<b>UNBRAC</b>	)	(LBS)	CSI (LC)	
FR-TO		FROM	TO		LENGTH	FR-TO			
1- 2	0 / 29	-77.3	-77.3	0.10(1)	10.00	7-3	-51 / 177	0.06 (4)	
2- 3	-513 / 176	-77.3	-77.3	0.70 (7)	6.25	3-6	-689 / 624	0.52(3)	
3- 4	-228 / 286	-77.3	-77.3	0.70 (7)	6.25	2-7	-166 / 509	0.19 (5)	
4- 5	-228 / 286	-77.3	-77.3	0.70 (7)	6.25				
6- 5	-245 / 222	0.0	0.0	0.45 (7)	6.25				
8- 2	-808 / 290	0.0	0.0	0.15 (4)	7.81				
8- 7	-706 / 505	-17.5	-17.5	0.25 (11	) 6.25				
7-6	-400 / 462	-17.5	-17.5	0.28 (11	) 6.25				
1									

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

**RECEIVED** TOWN OF MILTON MAR 29, 2017 17-4978 **BUILDING DIVISION**  **DESIGN CRITERIA** 

SPECIFIED LOADS: TOP CH.

LL = DL = LL = DL = 3.0 PSF 7.0 PSF = TOTAL LOAD 33.3

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

TOTAL WEIGHT = 2 X 83 = 166 lb

SCALE = 1:86.0

THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014

- CSA 086-09 - TPIC 2011

> (55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.47")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.47") CALCULATED VERT. DEFL.(TL) = L/999 (0.06")

CSI: TC=0.70 (2-3:7), BC=0.28 (6-7:11), WB=0.52 (3-6:3) , SSI=0.23 (2-3:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (4) (INPUT = 0.90 ) JSI METAL= 0.24 (4) (INPUT = 1.00 )



JOB NAME

Kott Lumber Uxbridge, Stouffville, ON, TW

TW0317-048

TRUSS NAME T16

QUANTITY PLY

2

TRUSS DESC

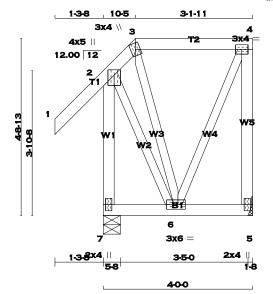
POBPES GREENPARK-LECCO RIDGE-BLOCK 327

DRWG NO.

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SCALE = 1:30.8

Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:41 2017 Page ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-wEMPVKcPApjZFNoGnTkkKhPjzJ1idZl9k8zYgJzcJJq



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR 3 2x4 DRY No.2 No.2 SPF SPF 2x4 DRY 2x4 DRY No 2 SPF No.2 5 2x4 DRY No.2 SPF ALL WEBS DRY No.2 2x3 EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

TYPE	PLATES	W	LEN	Υ	Χ
TMVW+p	MT20	4.0	5.0	1.75	2.00
TTW+m	MT20	3.0	4.0		
TMVW-t	MT20	3.0	4.0		
BMV1+p	MT20	2.0	4.0		
BMWWW-t	MT20	3.0	6.0		
BMV1+p	MT20	2.0	4.0		
	TMVW+p TTW+m TMVW-t BMV1+p BMWWW-t	TMVW+p MT20 TTW+m MT20 TMVW-t MT20 BMV1+p MT20 BMWWW-t MT20	TMVW+p MT20 4.0 TTW+m MT20 3.0 TMVW-t MT20 3.0 BMV1+p MT20 2.0 BMWWW-t MT20 3.0	TMVW+p         MT20         4.0         5.0           TTW+m         MT20         3.0         4.0           TMVW-t         MT20         3.0         4.0           BMV1+p         MT20         2.0         4.0           BMWWW-t         MT20         3.0         6.0	TMVW+p         MT20         4.0         5.0         1.75           TTW+m         MT20         3.0         4.0           TMVV+t         MT20         3.0         4.0           BMV1+p         MT20         2.0         4.0           BMWW-t         MT20         3.0         6.0

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

3EA	RINGS						
	FACTOR	RED	MAXIMUI	M FACTO	INPUT	REQRD	
	<b>GROSS RE</b>	ACTION	GROSS F	REACTIO	N	BRG	BRG
JΤ	VERT	HORZ	DOWN	HORZ	<b>UPLIFT</b>	IN-SX	IN-SX
5	171	0	215	0	-208	HANGER E	BY OTHERS
						MIN. SEAT	SIZE: 1-8
7	316	0	348	277	-99	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 5 FOR 208 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 7 FOR 150 LBS FACTORED UPLIFT

PROVIDE FOR 277 LBS FACTORED HORIZONTAL REACTION AT JOINT 7

UNFACTORED REACTIONS

	1ST LCASE	MAX./	MAX./MIN. COMPONENT REACTIONS								
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL				
5	120	82 / 0	0/0	0/0	111 / -174	39 / 0	0/0				
7	219	169 / 0	0/0	0/0	78 / -103	50 / 0	0/0				
HOF 7	RIZONTAL RE	ACTIONS 0/0	0/0	0/0	198 / -173	0/0	0 /0				

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 7

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (11)

CHC	DRDS				WEBS					
MAX.	FACTORED	FACTOR	RED				MAX. FACTO	ORED		
MEMB.	FORCE	VERT. LO.	AD LC	1 MAX	MAX.	MEMB.	FORCE	MAX		
	(LBS)	(PL	F) (	CSI (LC)	<b>UNBRAC</b>	)	(LBS)	CSI (LC)		
FR-TO		FROM	TO		LENGTH	FR-TO				
1- 2	0/38	-77.3	-77.3	0.11 (7)	10.00	3-6	-67 / 40	0.02 (7)		
2-3	-123 / 191	-77.3	-77.3	0.14 (7)	6.25	6- 4	-173 / 146	0.06 (6)		
3- 4	-35 / 123	-77.3	-77.3	0.13(1)	6.25	2-6	-159 / 215	0.04 (5)		
5- 4	-198 / 217	0.0	0.0	0.14 (7)	7.81					
7- 2	-333 / 106	0.0	0.0	0.09 (7)	7.81					
7.0	200 / 240	47.5	47.5	0.04 (5)	0.05					
7-6	-208 / 210			0.04 (5)	6.25					
6- 5	-34 / 87	-17.5	-17.5	0.02(7)	6.25					

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM},INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

## **DESIGN CRITERIA**

SPECIFIED LOADS: CH. PSF

LL = DL = LL = DL = AD = 3.0 PSF PSF 7.0 PSF TOTAL LOAD 33.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

TOTAL WEIGHT = 2 X 31 = 62 lb

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09

- TPIC 2011

DESIGN ASSUMPTIONS OVERHANG NOT TO BE ALTERED OR CUT

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19\*) CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00\*) ALLOWABLE DEFL.(TL)= L/360 (0.19\*) CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00\*)

CSI: TC=0.14 (2-3:7) , BC=0.04 (6-7:5) , WB=0.06 (4-6:6) , SSI=0.09 (3-4:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

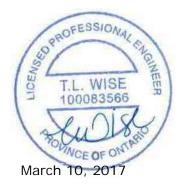
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.48 (3) (INPUT = 0.90) JSI METAL= 0.09 (3) (INPUT = 1.00)





READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

JOB NAME TRUSS NAME

Kott Lumber Uxbridge, Stouffville, ON, TW

T17

TW0317-048

QUANTITY PLY 2

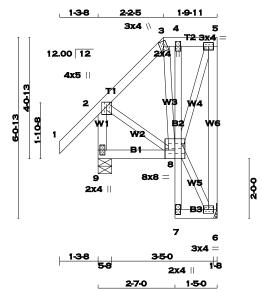
POBPES GREENPARK-LECCO RIDGE-BLOCK 327 TRUSS DESC.

DRWG NO.

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SCALE = 1:38.7

Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:41 2017 Page ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-wEMPVKcPApjZFNoGnTkkKhPi7J1gdYR9k8zYgJzcJJq



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR 3 DRY No.2 No.2 SPF 5 2 2x4 DRY No 2 2x4 No.2 9 8 2x4 DRY No.2 SPF SPF No.2 DRY 6 No.2 ALL WEBS EXCEPT

DRY: SEASONED LUMBER.

PLATES	table	is in	inches)

	TILO (lable i	3 III IIICIIC	21			
JT	TYPE	PLATES	W	LEN	Υ	X
2	TMVW+p	MT20	4.0	5.0	1.75	2.00
3	TTW+m	MT20	3.0	4.0		
4	TMV+p	MT20	2.0	4.0		
5	TMVW-t	MT20	3.0	4.0		
6	BMVW1-t	MT20	3.0	4.0		
7	BMV+p	MT20	2.0	4.0		
8	BVMWWWW	*-MT20	8.0	8.0	Edge	1.50
9	BMV1+p	MT20	2.0	4.0	-	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEA	RINGS						
	FACTOR	RED	MAXIMU	M FACT	INPUT	REQRD	
	<b>GROSS RE</b>	ACTION	GROSS I	REACTIO	N	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
6	190	0	248	274	-295	HANGER E	BY OTHERS
						MIN. SEAT	SIZE: 1-8
9	298	0	360	0	-133	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 6 FOR 295 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 9 FOR 150 LBS FACTORED UPLIFT

PROVIDE FOR 274 LBS FACTORED HORIZONTAL REACTION AT JOINT 6

### UNFACTORED REACTIONS

	1ST LCASE	MAX./N	IIN. COMPO	NENT REACTION	ONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
6	133	93 / 0	0/0	0/0	145 / -236	40 / 0	0/0		
9	206	158 / 0	0/0	0/0	157 / -126	48 / 0	0/0		
HOF	HORIZONTAL REACTIONS								
6		0/0	0/0	0/0	252 / -179	0/0	0 /0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 9

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (11)

СНС	RDS					WE	BS		
MAX.	FACTORED	FACTOR	RED				MAX. FACT	ORED	
MEMB.	FORCE	VERT. LO	AD LC1	1 MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PL	F) (	CSI (LC)	UNBRAC		(LBS)	CSI (LC)	
FR-TO	` '	FROM	ΤΌ	. ,	LENGTH	FR-TO	, ,	` ,	
1-2	0 / 38	-77.3	-77.3	0.11 (7)	10.00	3-8	-81 / 120	0.02 (6)	
2-3	-138 / 146	-77.3	-77.3	0.10 (7)	6.25	8-6	-263 / 421	0.08 (6)	
3-4	-92 / 118	-77.3	-77.3	0.03 (5)	6.25	8- 5	-32 / 141	0.03(1)	
4- 5	-90 / 116	-77.3	-77.3	0.03 (8)	6.25	2-8	-151 / 241	0.05 (6)	
6- 5	-185 / 49	0.0	0.0	0.19 (7)	7.81				
9- 2	-337 / 150	0.0	0.0	0.03 (4)	7.81				
9-8	-14 / 30	-17.5	-17.5	0.04 (11)	6.25				
7-8	0 / 13	0.0		0.01 (4)	10.00				
8- 4	-80 / 75	0.0			7.81				
7-6	0/1	-17.5		0.01 (11)	10.00				

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT WIND LOAD APPLIED IS DERIVED FROM REPERSING VELOCITY FRESTURE OF (\$3.0) FOR AL (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.

# **DESIGN CRITERIA**

SPEC	IFIED	LOAD:	S:		
TOP	CH.	LL :	=	23.3	PSF
				3.0	PSF
BOT	CH.	LL :	=	0.0	PSF
		DL :	=	7.0	PSF

= 33.3 TOTAL LOAD SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

TOTAL WEIGHT = 2 X 35 = 69 lb

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09

- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19") CALCULATED VERT. DEFL.(LL)= L/ 999 (0.00") ALLOWABLE DEFL.(TL)= L/360 (0.19") CALCULATED VERT. DEFL.(TL)= L/999 (0.00")

CSI: TC=0.19 (5-6:7), BC=0.04 (8-9:11), WB=0.08 (6-8:6), SSI=0.08 (5-6:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (6) (INPUT = 0.90) JSI METAL= 0.12 (6) (INPUT = 1.00)



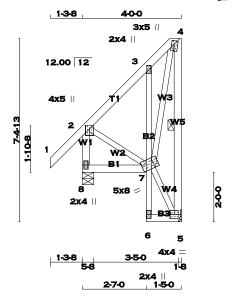


READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

POBPES GREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY PLY DRWG NO. Page 76 of 159 TRUSS DESC. 2 TW0317-048 TW0317-048 T18

Kott Lumber Uxbridge, Stouffville, ON, TW

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TOTAL WEIGHT = 2 X 36 = 72 lb

SCALE = 1:46.5

LUME	<u>BER</u>				
N. L. (	G. A. R	ULES			
CHOR	RDS	SIZE		LUMBER	DESCR.
1 -	4	2x4	DRY	No.2	SPF
5 -	4	2x4	DRY	No.2	SPF
8 -	2	2x4	DRY	No.2	SPF
8 -	7	2x4	DRY	No.2	SPF
6 -	3	2x3	DRY	No.2	SPF
6 -	5	2x4	DRY	No.2	SPF
ALL V	VEBS	2x3	DRY	No.2	SPF
EXCE	PT				

DRY: SEASONED LUMBER.

### PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Υ	X
2	TMVW+p	MT20	4.0	5.0	1.75	2.00
3	TMV+p	MT20	2.0	4.0		
4	TMVW+w	MT20	3.0	5.0	1.75	1.25
5	BMVW1-t	MT20	4.0	4.0		
6	BMV+p	MT20	2.0	4.0		
7	BVMWWW-w	MT20	5.0	8.0	2.75	2.25
8	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY **BUILDING DESIGNER** 

	KINGS						
	FACTOR	RED	MAXIMUI	M FACT	ORED	INPUT	REQRD
	GROSS RE	ACTION	GROSS F	REACTIO	N	BRG	BRG
JΤ	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
5	190	0	329	401	-482	HANGER E	BY OTHERS
						MIN. SEAT	SIZE: 1-8
3	298	0	415	0	-242	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 5 FOR 482 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 8 FOR 242 LBS FACTORED UPLIFT

PROVIDE FOR 401 LBS FACTORED HORIZONTAL REACTION AT JOINT 5

### UNFACTORED REACTIONS

	1ST LCASE	MAX./N	MIN. COMPO	NENT REACTION	SNC			
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
5	133	93 / 0	0/0	0/0	251 / -370	40 / 0	0/0	
8	206	158 / 0	0/0	0/0	294 / -204	48 / 0	0/0	
HOF	HORIZONTAL REACTIONS							
5		0/0	0/0	0/0	343 / -236	0/0	0 /0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 8

## **BRACING**

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 4-5. DBS = 20-0-0 . CBF = 22 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING TOTAL LOAD CASES: (11)

CHC	RDS				WEBS					
MAX.	FACTORED	FACTOR	RED				MAX. FACTO	RED		
MEMB.	FORCE	VERT. LO	AD LC	1 MAX	MAX.	MEMB.	FORCE	MAX		
	(LBS)	(PL	F) (	CSI (LC)	UNBRAC		(LBS)	CSI (LC)		
FR-TO		FROM	TO		LENGTH	FR-TO		, ,		
1- 2	0 / 38	-77.3	-77.3	0.11 (7)	10.00	7- 5	-332 / 603	0.12 (6)		
2-3	-311 / 245	-77.3	-77.3	0.09(7)	6.25	7- 4	-134 / 257	0.06 (5)		
3- 4	-189 / 247	-77.3	-77.3	0.11 (7)	6.25	2-7	-231 / 379	0.07 (6)		
5- 4	-175 / 0	0.0	0.0	0.33 (8)	6.25					
8- 2	-392 / 259	0.0	0.0	0.04 (5)	7.81					
8- 7	-14 / 30	-17.5	-17.5	0.04 (11)	6.25					
6- 7	0 / 13	0.0	0.0	0.01 (4)	10.00					
7-3	-245 / 333	0.0	0.0	0.07(7)	7.81					
6- 5	-1 / 2	-17.5	-17.5	0.01 (11)	10.00					

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT (40-0-0) F-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK
COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL
WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON
(OPEN TERRAIN). AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT **CONTAINS SPECIFICATIONS AND CRITERIA USED** IN THE DESIGN OF THIS COMPONENT.

**RECEIVED** TOWN OF MILTON MAR 29, 2017 17-4978 **BUILDING DIVISION**  **DESIGN CRITERIA** 

SPECIFIED LOADS: LL = DL = LL = DL = AD = CH. 3.0 PSF 7.0 PSF

TOTAL LOAD 33.3

### SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09

- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19") CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.33 (4-5:8), BC=0.07 (3-7:7), WB=0.12 (5-7:6) , SSI=0.10 (4-5:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.64 (5) (INPUT = 0.90) JSI METAL= 0.16 (5) (INPUT = 1.00)



JOB NAME TRUSS NAME T19

TW0317-048

Kott Lumber Uxbridge, Stouffville, ON, TW

QUANTITY PLY JOB PR-GREENPARK-LECCO RIDGE-BLOCK 327 TRUSS DESC

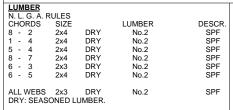
DRWG NO.

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1-3-8 400 3x6 / 2x4 | 12.00 12 4x5 || 1-10-8 5x8 2x4 || 4x4 6 2x4 🛚

SCALE = 1:49.3



### PLATES (table is in inches)

TYPE	PLATES	W	LEN	Υ	Χ
TMVW+p	MT20	4.0	5.0	1.75	2.00
TMV+p	MT20	2.0	4.0		
TMVW-t	MT20	3.0	6.0	1.50	2.50
BMVW1-t	MT20	4.0	4.0		
BMV+p	MT20	2.0	4.0		
BVMWWW-w	MT20	5.0	8.0	2.75	2.25
BMV1+p	MT20	2.0	4.0		
	TMVW+p TMV+p TMVW-t BMVW1-t BMV+p BVMWWW-w	TMVW+p MT20 TMV+p MT20 TMVW+t MT20 BMVW1-t MT20 BMV+p MT20 BVMWWW-w MT20	TMVW+p MT20 4.0 TMV+p MT20 2.0 TMVW+t MT20 3.0 BMVW1-t MT20 4.0 BMV+p MT20 4.0 BVMWWW-w MT20 5.0	TMVW+p MT20 4.0 5.0 TMV+p MT20 2.0 4.0 TMVW+t MT20 3.0 6.0 BMVW1+t MT20 4.0 4.0 BMV+p MT20 2.0 4.0 BVMWWW-w MT20 5.0 8.0	TMVW+p MT20 4.0 5.0 1.75 TMV+p MT20 2.0 4.0 TMVW+t MT20 3.0 6.0 1.50 BMVW1+t MT20 4.0 4.0 BMV+p MT20 2.0 4.0 BVMWWW-w MT20 5.0 8.0 2.75

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

### DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

2.7.0

1.50

BE/	RINGS						
	FACTO	RED	MAXIMU	JM FACT	INPUT	REQRD	
	GROSS R	EACTION	GROSS	REACTIO	N	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
8	298	0	415	0	-242	5-8	5-8
5	190	0	329	401	-482	HANGER	BY OTHERS
						MIN. SEA	T SIZE: 1-12

PROVIDE ANCHORAGE AT BEARING JOINT 8 FOR 242 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 5 FOR 482 LBS FACTORED UPLIFT

PROVIDE FOR 401 LBS FACTORED HORIZONTAL REACTION AT JOINT 5

### UNFACTORED REACTIONS

	1ST LCASE	MAX./N	IIN. COMPO	NENT REACTION	ONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
8	206	158 / 0	0/0	0/0	294 / -204	48 / 0	0/0		
5	133	93 / 0	0/0	0/0	251 / -370	40 / 0	0/0		
HOR	HORIZONTAL REACTIONS								
5		0/0	0/0	0/0	343 / -236	0/0	0 /0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 8

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 4-5. DBS = 20-0-0 . CBF = 22 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING TOTAL LOAD CASES: (11)

СНС	ORDS					W E	BS	
MAX.	FACTORED	FACTO	RED				MAX. FACTO	RED
MEMB.	FORCE	VERT. LC	AD LC	1 MAX	MAX.	MEMB.	FORCE	MAX
	(LBS)	(PL	.F) (		UNBRAC		(LBS)	CSI (LC)
FR-TO		FROM	TO		LENGTH	FR-TO		
8- 2	-392 / 259	0.0	0.0	0.04 (5)	7.81	2- 7	-231 / 379	0.07 (6)
1- 2	0/38	-77.3	-77.3	0.11 (7)	10.00	7- 5	-332 / 603	0.12 (6)
2- 3	-311 / 245	-77.3	-77.3	0.09 (7)	6.25	7- 4	-134 / 257	0.06 (5)
3- 4	-189 / 247	-77.3	-77.3	0.11 (7)	6.25			
5- 4	-175 / 0	0.0	0.0	0.33 (8)	6.25			
8- 7	-14 / 30	-17.5		0.04 (11)	6.25			
6- 7	0 / 13	0.0	0.0	0.01 (4)	10.00			
7- 3	-245 / 333	0.0		0.07 (7)	7.81			
6- 5	-1 / 2	-17.5	-17.5	0.01 (11)	10.00			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT (40-0-0) F-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK
COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL
WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON
(OPEN TERRAIN). AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

**RECEIVED** TOWN OF MILTON MAR 29, 2017 17-4978 **BUILDING DIVISION** 

### **DESIGN CRITERIA**

۱	SPEC	IFIED	LOAI	DS:		
					23.3	PSF
١			DL	=	3.0	PSF
١	BOT	CH.	LL	=	0.0	PSF
١			DL	=	7.0	PSF
١	TOTA	L LO	AD	=	33.3	PSF

### 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

TOTAL WEIGHT = 4 X 36 = 146 lb

# THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014

- CSA 086-09 - TPIC 2011

> (55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19") CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.33 (4-5:8), BC=0.07 (3-7:7), WB=0.12 (5-7:6) , SSI=0.10 (4-5:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.64 (5) (INPUT = 0.90 ) JSI METAL= 0.16 (5) (INPUT = 1.00 )



JOB NAME TRUSS NAME

Kott Lumber Uxbridge, Stouffville, ON, TW

TW0317-048

T20

QUANTITY PLY

2

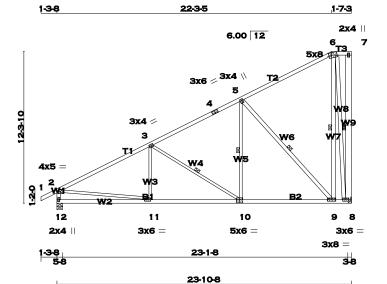
JOB PESC TR-GREENPARK-LECCO RIDGE-BLOCK 327 TRUSS DESC

DRWG NO.

Page 78 of 159 TW0317-048

SCALE = 1:93.4

Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:42 2017 Page ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-ORwoifd1x7rQtXNSLBGztuynFjIEMo0Jzoj5ClzcJJp



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR 4 2x4 DRY No.2 No.2 SPF SPF 2x4 6 8 2x4 DRY No 2 SPF 7 2x6 No.2 12 -2x4 DRY No.2 SPF SPF SPF 10 No.2 10-8 No.2 ALL WEBS EXCEPT SPF DRY No.2 SPF SPF 9 DRY No.2 No.2 2x4 6 -8 2x4 DRY No.2

DRY: SEASONED LUMBER

PL/	ATES (table	is in inches	s)			
JT	TYPE	PLATES	W	LEN	Υ	Χ
2	TMVW-p	MT20	4.0	5.0	1.50	2.25
3	TMWW-t	MT20	3.0	4.0	1.50	1.75
4	TS-t	MT20	3.0	6.0		
5	TMWW+t	MT20	3.0	4.0	1.50	0.75
6	TTWW-m	MT20	5.0	8.0	2.25	2.75
7	TMV+p	MT20	2.0	4.0		
8	BMVW1-t	MT20	3.0	6.0		
9	BMWW-t	MT20	3.0	8.0		
10	BSWW-I	MT20	5.0	6.0	3.25	3.00
11	BMWW-t	MT20	3.0	6.0	1.50	1.75
12	BMV1+p	MT20	2.0	4.0		

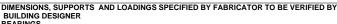
A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

T.L. WISE

100083566

NCE OF ON

March 10, 2017



BEA	KINGS						
	FACTO	RED	MAXIMU	M FACT	ORED	INPUT	REQRD
	GROSS R	EACTION	GROSS	REACTION	N	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
8	1132	0	1191	0	-685	HANGER	BY OTHERS
						MIN. SEA	AT SIZE: 3-8
12	1237	0	1272	725	-573	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 8 FOR 685 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 12 FOR 573 LBS FACTORED UPLIFT

ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES. SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 725 LBS FACTORED HORIZONTAL REACTION AT JOINT 12

### UNFACTORED REACTIONS

	1ST LCASE	MAX./N	MIN. COMPO	NENT REACTION	ONS		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
8	795	556 / 0	0/0	0/0	146 / -643	239 / 0	0/0
12	866	619/0	0/0	0/0	86 / -568	247 / 0	0/0
HOR 12	IZONTAL REA	ACTIONS 0/0	0/0	0/0	518 / -122	0/0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 12

<u>BRACING</u> TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.61 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

- 1 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 7-8, 3-10, 5-10, 6-9. DBS = 20-0-0 . CBF = 81 LBS.
- 1 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 5-9. DBS = 14-0-0 . CBF = 90 LBS. 1 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 6-8. DBS = 12-0-0 . CBF = 82 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL (S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING TOTAL LOAD CASES: (11)

СН	ORDS					W E	BS		
MA.	X. FACTORED	FACTOR	RED				MAX. FACTO	DRED	
MEMB.	FORCE	VERT. LO	AD LC1	I MAX	MAX.	MEMB	. FORCE	MAX	
	(LBS)	(PL	.F) (	CSI (LC)	UNBRAG	0	(LBS)	CSI (LC)	
FR-TO	, ,	FROM	TO	,	LENGTH	FR-TO		` '	
1-2	0 / 23	-77.3	-77.3	0.10(1)	10.00	11-3	-21 / 160	0.04 (11)	
2-3	-1574 / 682	-77.3	-77.3	0.62 (7)	4.61	3-10	-688 / 517	0.35 (3)	
3-4	-1002 / 504	-77.3	-77.3	0.59 (7)	5.76	10- 5	-189 / 505	0.11 (1)	
4- 5	-1002 / 504	-77.3	-77.3	0.59 (7)	5.76	5- 9	-1085 / 771	0.73 (3)	
5-6	-270 / 231	-77.3	-77.3	0.54 (7)	6.25	9-6	-464 / 898	0.39 (7)	
6- 7	-92 / 235	-77.3	-77.3	0.06 (7)	6.25	6-8	-1144 / 601	0.96 (3)	
8- 7	-65 / 49	0.0	0.0	0.42 (7)	6.25	2-11	-455 / 1448	0.51 (7)	
12-2	-1217 / 610	0.0	0.0	0.12(1)	7.30				
12-11	-707 / 162	-17.5	-17.5	0.23 (11	) 6.25				
11-10_	-792 / 1445	<u>-17.</u> 5	-17.5	0.34(1)	6.25				
10-9	44' / 8			(	6.25				
9-8	/ 2	17	- 5	0.1 11	) 6.25				
- 6	Ŧ <b>. T</b>						D		
WIND	APPEED						Y PRESSURE		
{40-0-0	D) FT-IN-SX REI	FERENCE H	HEIGHT	ABOVE	GRADE	AND US	SING EXTERMA	anireakiviii	Т

COE WIN {OPE FRO READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED

IN THE DESIGN OF THIS COMPONENT.

**D** EXTERMAL PORKMILTON ING SYSTEM).INTERNAL MAY BIMIAR ATO DOM'T EAST (0-0) FT-IN-SX AWAY 17-4978

**BUILDING DIVISION** 

TOTAL WEIGHT = 2 X 143 = 286 lb

**DESIGN CRITERIA** SPECIFIED LOADS:

LL = DL = LL = DL = AD = CH. PSF 3.0 PSF 7.0 PSF TOTAL LOAD 33.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09

- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.80°) CALCULATED VERT. DEFL.(LL)= L/ 999 (0.06°) ALLOWABLE DEFL.(TL)= L/360 (0.80°) CALCULATED VERT. DEFL.(TL)= L/999 (0.14°)

CSI: TC=0.62 (2-3:7), BC=0.34 (10-11:1), WB=0.96 (6-8:3), SSI=0.24 (2-3:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

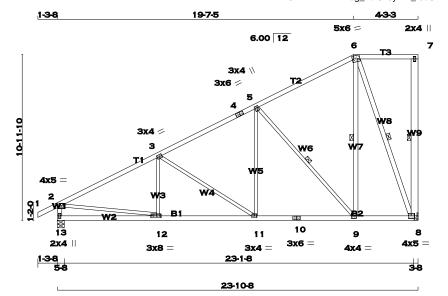
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (11) (INPUT = 0.90) JSI METAL= 0.41 (2) (INPUT = 1.00) JOB NAME TRUSS NAME QUANTITY PLY DRWG NO. JOBPESC TR-GREENPARK-LECCO RIDGE-BLOCK 327 2 TW0317-048 T21 Kott Lumber Uxbridge, Stouffville, ON, TW

Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:43 2017 Page ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-sdUAw?dfiRzHVhyfvunCQ6U\_k7ft5lcSCSSfkCzcJJo



LUMBER				
N. L. G. A. F	RULES			
CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 6	2x4	DRY	No.2	SPF
6 - 7	2x4	DRY	No.2	SPF
8 - 7	2x6	DRY	No.2	SPF
13 - 2	2x4	DRY	No.2	SPF
13 - 10	2x4	DRY	No.2	SPF
10 - 8	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
9 - 6	2x4	DRY	No.2	SPF
6 - 8	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PL	ATES (table	is in inches	s)			
JT	TYPE	PLATES	W	LEN	Υ	Χ
2	TMVW-p	MT20	4.0	5.0	1.50	2.25
3	TMWW-t	MT20	3.0	4.0	1.50	1.75
4	TS-t	MT20	3.0	6.0		
5	TMWW+t	MT20	3.0	4.0	1.50	0.75
6	TTWW-m	MT20	5.0	6.0	2.25	1.00
7	TMV+p	MT20	2.0	4.0		
8	BMVW1-t	MT20	4.0	5.0	2.00	2.25
9	BMWW-t	MT20	4.0	4.0	2.00	1.75
10	BS-t	MT20	3.0	6.0		
11	BMWW-t	MT20	3.0	4.0		
12	BMWW-t	MT20	3.0	8.0	1.50	3.50
13	BMV1+p	MT20	2.0	4.0		

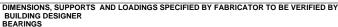
A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

T.L. WISE

100083566

NCEOFON

March 10, 2017



	FACTOR	RED	MAXIMU	M FACT	ORED	INPUT	REQRD
	GROSS RE	ACTION	GROSS I	REACTIC	N	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
8	1132	0	1188	0	-667	HANGER E	BY OTHERS
						MIN. SEAT	SIZE: 3-8
13	1237	0	1274	647	-591	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 8 FOR 667 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 13 FOR 591 LBS FACTORED UPLIFT

ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES. SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 647 LBS FACTORED HORIZONTAL REACTION AT JOINT 13

# UNFACTORED REACTIONS

	1ST LCASE	MAX./	MIN. COMPO	<u>NENT REACTIO</u>	ONS		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
8	795	556 / 0	0/0	0/0	140 / -630	239 / 0	0/0
13	866	619/0	0/0	0/0	91 / -581	247 / 0	0/0
HOR	IZONTAL REA	ACTIONS					
13		0/0	0/0	0/0	462 / -111	0/0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 13

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.80 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

- 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 7-8, 6-9. DBS = 20-0-0 . CBF = 48 LBS. 1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 5-9. DBS = 16-0-0 . CBF = 88 LBS. 1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 6-8. DBS = 14-0-0 . CBF = 89 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING TOTAL LOAD CASES: (11)

CH	IORDS				W E	BS		
MA.	X. FACTORED	FACTORED				MAX. FACTO	DRED	
MEMB.	FORCE	VERT. LOAD LO	C1 MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PLF)	CSI (LC)	<b>UNBRAC</b>	)	(LBS)	CSI (LC)	
FR-TO	, ,	FROM TO	,	LENGTH	FR-TO	, ,	` ,	
1- 2	0 / 23	-77.3 -77.3	3 0.10 (1)	10.00	12-3	-59 / 161	0.03 (11)	
2- 3	-1587 / 730	-77.3 -77.3	3 0.51 (7)	4.80	3-11	-556 / 431	0.66 (3)	
3- 4	-1122 / 599	-77.3 -77.3	3 0.49 (7)	5.65	11-5	-155 / 418	0.17 (7)	
4- 5	-1122 / 599	-77.3 -77.3	3 0.49 (7)	5.65	5-9	-929 / 666	0.64 (3)	
5-6	-502 / 372	-77.3 -77.3	3 0.44 (7)	6.25	9-6	-420 / 798	0.25 (7)	
6- 7	-82 / 209	-77.3 -77.3	3 0.24 (1)	6.25	6-8	-1067 / 591	0.74(3)	
8- 7	-174 / 133	0.0 0.0	0.34 (7)	6.25	2-12	-512 / 1451	0.40 (7)	
13- 2	-1225 / 624	0.0 0.0	0.12(1)	7.29				
13-12	-629 / 147	-17.5 -17.5	5 0.18 (11	) 6.25				
12-11	-809 / 1465	-17.5 -17.5	5 0.31 (1)	6.25				
11-10	-490 <sub>/</sub> /999	<u>-17.</u> 5 <u>-17.</u> 5	0.23 (1)	6.25				
10-9	49' / 99			6.25				
9-8	/ 3:	17 - 5	5 0.1 11	) 6.25				
						D		
WIND	APPELED		OM REFER	RENCE V	ELOCIT	Y PRESSURE		דאָבוּ
	0} FT-IN-SX REF	ERENCE HEIGH	IT ABOVE	GRADE	AND US			
COE	READ ALL NO	TES ON THIS F	AGE AN	D ON TH	IF		ΓΕΜ}.INTERN	
WIN	ENGINEEDING	NOTE PAGE E	ND 4 TI	JE NOTI			<b>4078</b> /4 <b>279</b>  P <b>20</b> 01	
OPE	ENGINEERING	NOTEPAGE	INT-I. II	15 NO 11	FAGE	LEAST {0-0	FT-IN-SX A	NAY

EAST (0-0) FT-IN-SX AWAY 17-4978

**BUILDING DIVISION** 

IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED

IN THE DESIGN OF THIS COMPONENT.

TOTAL WEIGHT = 2 X 131 = 261 lb

Page 79 of 159

SCALE = 1:76.3

TW0317-048

**DESIGN CRITERIA** 

SPECIFIED LOADS: LL = DL = LL = DL = AD = CH. 3.0 PSF 7.0 PSF TOTAL LOAD 33.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09

- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.80°) CALCULATED VERT. DEFL.(LL)= L/ 999 (0.06°) ALLOWABLE DEFL.(TL)= L/360 (0.80°) CALCULATED VERT. DEFL.(TL)= L/999 (0.11°)

CSI: TC=0.51 (2-3:7), BC=0.31 (11-12:1), WB=0.74 (6-8:3), SSI=0.21 (2-3:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (12) (INPUT = 0.90) JSI METAL= 0.41 (2) (INPUT = 1.00) JOB NAME TRUSS NAME

Kott Lumber Uxbridge, Stouffville, ON, TW

T22

TW0317-048

QUANTITY 2

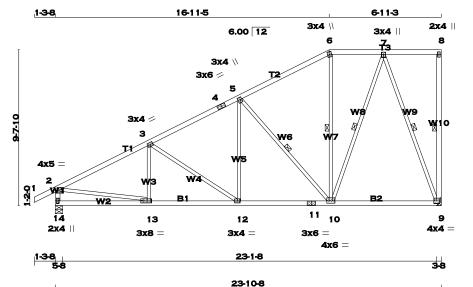
JOB PESC PREENPARK-LECCO RIDGE-BLOCK 327 TRUSS DESC

DRWG NO

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SCALE = 1:71.3

Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:43 2017 Page ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-sdUAw?dfiRzHVhyfvunCQ6UzY7fF5IDSCSSfkCzcJJo



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR 4 2x4 DRY No.2 No.2 SPF SPF 6 2x4 DRY No 2 SPF 8 2x4 No.2 14 -2x4 DRY No.2 SPF 14 -11 -SPF No.2 DRY 9 2x4 No.2 ALL WEBS EXCEPT 2x3 DRY No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)
-----------------------------

JT	TYPE	PLATES	W	LEN	Υ	Χ	
2	TMVW-p	MT20	4.0	5.0	1.50	2.25	
3	TMWW-t	MT20	3.0	4.0	1.50	1.75	
4	TS-t	MT20	3.0	6.0			
5	TMWW+t	MT20	3.0	4.0	1.75	0.75	
6	TTW+m	MT20	3.0	4.0	2.00	1.25	
7	TMWW+t	MT20	3.0	4.0	1.75	1.50	
8	TMV+p	MT20	2.0	4.0			
9	BMVW1-t	MT20	4.0	4.0			
10	BMWWW-t	MT20	4.0	6.0	1.75	3.00	
11	BS-t	MT20	3.0	6.0			
12	BMWW-t	MT20	3.0	4.0			
13	BMWW-t	MT20	3.0	8.0	1.50	3.25	
14	BMV1+p	MT20	2.0	4.0			

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

T.L. WISE

100083566

NCE OF ON

March 10, 2017

# DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS

DEA	BEARINGS											
	FACTO	RED	MAXIMU	IM FACT	ORED	INPUT	REQRD					
	GROSS R	EACTION	GROSS	REACTIO	N	BRG	BRG					
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX					
9	1132	0	1187	0	-651	HANGER	R BY OTHERS					
						MIN. SEA	AT SIZE: 3-8					
14	1237	0	1276	568	-607	5-8	5-8					

PROVIDE ANCHORAGE AT BEARING JOINT 9 FOR 651 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 14 FOR 607 LBS FACTORED UPLIFT

ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES. SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 568 LBS FACTORED HORIZONTAL REACTION AT JOINT 14

### UNFACTORED REACTIONS

	1ST LCASE	MAX./	MIN. COMPON	<b>IENT REACTIO</b>	ONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL			
9	795	556 / 0	0/0	0/0	136 / -618	239 / 0	0/0			
14	866	619/0	0/0	0/0	96 / -593	247 / 0	0/0			
HOR	HORIZONTAL REACTIONS									
14		0/0	0/0	0/0	406 / -100	0/0	0 /0			

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 14

<u>BRACING</u> TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.94 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

- 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 8-9, 5-10, 6-10, 7-10. DBS = 20-0-0. CBF = 90 LBS1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 7-9. DBS = 14-0-0 . CBF = 92 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4,

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

WIN {OPE FRO

LOADING TOTAL LOAD CASES: (11)

	10 M2 20/18 0/1020. (11)									
	CHORDS WEBS									
MAX	K. FACTORED	FACTO	RED			MAX. FACTO	RED			
MEMB.	FORCE	VERT. LO	AD LC1 MAX	MAX.	MEMB.	FORCE	MAX			
	(LBS)		.F) CSI (LC)			(LBS)	CSI (LC)			
FR-TO		FROM	TO	LENGTH						
1- 2	0 / 23	-77.3	-77.3 0.10 (1)	10.00	13- 3	-104 / 167	0.03 (7)			
2- 3	-1604 / 767	-77.3	-77.3 0.41 (7)	4.94	3-12	-420 / 340	0.34 (3)			
3- 4	-1247 / 688	-77.3	-77.3 0.40 (7)	5.51	12-5	-129 / 322	0.10 (7)			
4- 5	-1247 / 688	-77.3	-77.3 0.40 (7)	5.51	5-10	-764 / 569	0.38 (3)			
5- 6	-734 / 502	-77.3	-77.3 0.36 (7)	6.25	10-6	-13 / 81	0.02 (11)			
6- 7	-638 / 527	-77.3	-77.3 0.18 (7)	6.25	10- 7	-350 / 717	0.24 (7)			
7-8	-72 / 183	-77.3	-77.3 0.15 (1)	6.25	7-9 -	1098 / 641	0.77 (3)			
9-8	-112 / 86	0.0	0.0 0.52 (7)	6.25	2-13	-559 / 1457	0.32(1)			
14- 2	-1233 / 635	0.0	0.0 0.12 (1)	7.27						
14-13	-550 / 131	-17.5	-17.5 0.14 (11)	6.25						
13-12	<u>-8</u> 16/1474	<u>-17.</u> 5	-17.5 0.29 (1)	6.25						
12-11	57 / 1			6.25						
11-10	/ 1	17 17	- 5 0.2 1) - 5 0.2 11)	6.25						
10-9	-1 / 3	17	- 5 0.2 11)	6.25						
E						RI	ECEIVED			
WIND	OAD APPLIED	IS DEBIVE	D EDOM DEEED	ENCE V	EI OCITY		NEOF MIETON			
[40 (										
COE										
COL	ENGINEEDING	NOTE DA	CE END_1 TH	IE NOTE	DAGE	S ING SY	*H'Y' / Z' Y' , '-Z' \U' / Y Y			

MAY BE LOCATED ON EAST (0-0) F7-119378 WAY

**BUILDING DIVISION** 

ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE

IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED

IN THE DESIGN OF THIS COMPONENT.

TOTAL WEIGHT = 2 X 117 = 234 lb

# **DESIGN CRITERIA**

SPEC	IFIED	LOA	os:		
TOP	CH.	LL	=	23.3	PSF
		DL	=	3.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.0	PSF
TOTA		۸ ا		22.2	DOD

### SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

### THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09

- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.80°) CALCULATED VERT. DEFL.(LL)= L/ 999 (0.06°) ALLOWABLE DEFL.(TL)= L/360 (0.80°) CALCULATED VERT. DEFL.(TL)= L/999 (0.11°)

CSI: TC=0.52 (8-9:7), BC=0.29 (12-13:1), WB=0.77 (7-9:3), SSI=0.18 (2-3:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

### NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

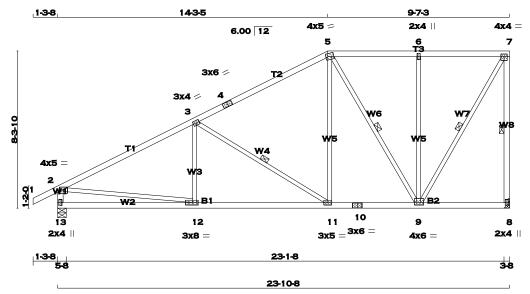
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (13) (INPUT = 0.90) JSI METAL= 0.41 (2) (INPUT = 1.00)

POBPES GREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY DRWG NO. TRUSS DESC. 2 TW0317-048 **T23** Kott Lumber Uxbridge, Stouffville, ON, TW

Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:43 2017 Page ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-sdUAw?dfiRzHVhyfvunCQ6Uxg7eP5J6SCSSfkCzcJJo



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR 4 5 7 DRY No.2 2x4 DRY No 2 7 2x4 No.2 13 -2x4 DRY No.2 SPF No.2 10 DRY 10 -SPF 8 No.2 ALL WEBS EXCEPT 2x3

DRY: SEASONED LUMBER.

PL	PLATES (table is in inches)												
JT	TYPE	PLATES	W	LEN	Υ	X							
2	TMVW-p	MT20	4.0	5.0	1.50	2.25							
3	TMWW-t	MT20	3.0	4.0	1.50	1.75							
4	TS-t	MT20	3.0	6.0									
5	TTWW-m	MT20	4.0	5.0	1.75	1.25							
6	TMW+w	MT20	2.0	4.0									
7	TMVW-t	MT20	4.0	4.0	1.50	2.00							

1.75 1.25 .50 2.00 MT20 BMV1+p 4.0 BMWWW-t BS-t MT20 MT20 4.0 6.0 1.50 1.50 10 3.0 6.0 BMWW-t 3.0 MT20 5.0 12 13 BMWW-t 1.50 3.50 BMV1+p MT20 4.0

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED

DIMENSIONS, SUPPORTS	AND LOADINGS	SPECIFIED I	BY FABRICATO	R TO BE VERIFIEI	DΒ
BUILDING DESIGNER					
BEARINGS					

	FACTO	RED	MAXIMU	M FACT	INPUT	REQRD						
	GROSS R	EACTION	GROSS	REACTIO	N	BRG	BRG					
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX					
8	1132	0	1185	0	-637	HANGER E	BY OTHERS					
						MIN. SEAT	SIZE: 3-8					
13	1237	0	1277	490	-621	5-8	5-8					

PROVIDE ANCHORAGE AT BEARING JOINT 8 FOR 637 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 13 FOR 621 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES. SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 490 LBS FACTORED HORIZONTAL REACTION AT JOINT 13

### UNFACTORED REACTIONS

	1ST LCASE	MAX./	MIN. COMPON	IENT REACTION	ONS		
JΤ	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
3	795	556 / 0	0/0	0/0	132 / -608	239 / 0	0/0
13	866	619/0	0/0	0/0	100 / -603	247 / 0	0/0
HOR	IZONTAL REA	ACTIONS					
13		0/0	0/0	0/0	350 / -89	0/0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 13

<u>BRACING</u> TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.58 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 7-8. DBS = 12-0-0 . CBF = 82 LBS. 1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 3-11, 5-9, 7-9. DBS = 20-0-0 . CBF = 87

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX, UNBRACED LENGTH COLUMN OF THE TABLE BELOW

(OPE

LOADING TOTAL LOAD CASES: (11)

		ORDS					W E			
	MA)	X. FACTORED	FACTOR					MAX. FACTO	RED	
	MEMB.	FORCE	VERT. LOA	AD LC1	I MAX			FORCE	MAX	
		(LBS)	(PLF	=) (	CSI (LC)	UNBRAC	0	(LBS)	CSI (LC)	
	FR-TO		FROM 1	ГО		LENGTH	FR-TO			
1	1-2	0 / 23	-77.3	-77.3	0.10(1)	10.00	12-3	-29 / 175	0.04 (11)	
l	2-3	-1598 / 792	-77.3	-77.3	0.70 (7)	4.58	3-11	-736 / 560	0.34(3)	
l	3-4	-981 / 596	-77.3	-77.3	0.67 (7)	5.61	11-5	-229 / 494	0.31 (7)	
l	4- 5	-981 / 596	-77.3	-77.3	0.67 (7)	5.61	5-9	-504 / 280	0.29(3)	
l	5-6	-611 / 484	-77.3	-77.3	0.27 (7)	6.25	9-6	-483 / 372	0.65 (3)	
l	6- 7	-611 / 484	-77.3	-77.3	0.27(7)	6.25	9- 7	-636 / 1166	0.37 (7)	
l	8- 7	-1149 / 660	0.0	0.0	0.46(7)	5.97	2-12	-564 / 1455	0.56 (7)	
l	13-2	-1225 / 656	0.0	0.0	0.12(1)	7.29				
l										
l	13-12	-472 / 116	-17.5	-17.5	0.23 (11	) 6.25				
l	12-11	-785 / 1473	-17.5	-17.5	0.34(1)	6.25				
l	11-10	<u>-3</u> 76 / 8 <u>56</u>	<u>-17.</u> 5	-17.5	0.20(1)	6.25				
l	10-9	37 / 8			(	6.25				
l	9-8	/ 1:	17	- 5	0.0 11	) 6.25				
l										
l	. F							D	CCEN/E	
l	WIND	APPETED			M REFER	RENCE V	ELOCITY	PRESSURE	DE( E0)\PE	FΙΔ
l		)} FT-IN-SX REF	FERENCE H	EIGHT	ABOVE	GRADE.	AND USI			
l	COE	READ ALL NO	TES ON TH	IIC D/	GE AN		IE		EM}.INTERN	
l	VVIIV	ENCINEEDING						C MAY B	40R4Z9P20	<b>J</b> 17

ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE

IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED

IN THE DESIGN OF THIS COMPONENT.

ING SYSTEM) INTERNAL MAY BINIAR AZ 9 D 2017 EAST (0-0) FT-IN-SX AWAY 17-4978

**BUILDING DIVISION** 

TOTAL WEIGHT = 2 X 111 = 221

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SCALE = 1:60.9

TW0317-048

# **DESIGN CRITERIA**

SPECIFIED LOADS: LL = DL = LL = DL = AD = CH. 3.0 PSF 7.0 PSF TOTAL LOAD 33.3

### SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

### THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09

- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.80°) CALCULATED VERT. DEFL.(LL)= L/ 999 (0.06°) ALLOWABLE DEFL.(TL)= L/360 (0.80°) CALCULATED VERT. DEFL.(TL)= L/999 (0.13°)

CSI: TC=0.70 (2-3:7), BC=0.34 (11-12:1), WB=0.65 (6-9:3), SSI=0.24 (2-3:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

### NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (7) (INPUT = 0.90) JSI METAL= 0.42 (2) (INPUT = 1.00)



JOB NAME TRUSS NAME QUANTITY PLY TRUSS DESC 2 TW0317-048 T24

1220

JOB PESC PREENPARK-LECCO RIDGE-BLOCK 327

DRWG NO.

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SCALE = 1:51.9

Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:44 2017 Page 1 ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-Kp2Y7LeHTk586qXrTclRyJ18DX?Jqo0cQ6CCGezcJJh

11-8-8

6.00 12 3x4 < 5 3 4x5 < 4x5 = 6 1412 B2 W8 W2 R1 9 10 8 11 3x6 = 2x4 || 3x8 = 4x6 = 3x8 = 23-3-8 58 18

LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR 2x4 2x4 DRY No.2 No.2 SPF SPF DRY 12 -7 -12 -2x4 DRY No 2 SPF 2x4 No.2 9 2x4 DRY No.2 SPF DRY No.2 SPF ALL WERS 2x3 DRY No.2 SPF EXCEPT

DRY: SEASONED LUMBER

Kott Lumber Uxbridge, Stouffville, ON, TW

<u>1-3-</u>8

PLATES (table is in inches)

	LATEO (tubic is in inches)											
JΤ	TYPE	PLATES	W	LEN	Υ	Χ						
2	TMVW-p	MT20	4.0	5.0	1.50	2.25						
3	TMWW-t	MT20	3.0	4.0	1.50	1.75						
4	TTW+p	MT20	3.0	4.0								
5	TMWW-t	MT20	3.0	4.0	1.50	1.75						
6	TMVW-t	MT20	4.0	5.0	1.50	2.25						
7	BMV1+p	MT20	2.0	4.0								
8	BMWW-t	MT20	3.0	8.0	1.50	2.25						
9	BS-t	MT20	3.0	6.0								
10	BMWWW-t	MT20	4.0	6.0								
11	BMWW-t	MT20	3.0	8.0	1.50	3.25						
12	BMV1+p	MT20	2.0	4.0								

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

23-10-8

BEA	RINGS						
	FACTOR	RED	MAXIMU	M FACT	INPUT	REQRD	
	<b>GROSS RE</b>	ACTION	GROSS I	REACTIO	BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
12	1237	0	1260	201	-558	5-8	5-8
7	1132	0	1157	0	-491	HANGER E	SY OTHERS SIZE: 1-8

PROVIDE ANCHORAGE AT BEARING JOINT 12 FOR 558 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 7 FOR 491 LBS FACTORED UPLIFT UPLIFT

ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES. SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 201 LBS FACTORED HORIZONTAL REACTION AT JOINT 12

UNFACTORED REACTIONS

	1ST LCASE	MAX./	MIN. COMPON	ONS			
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
12	866	619/0	0/0	0/0	57 / -558	247 / 0	0/0
7	795	556 / 0	0/0	0/0	62 / -504	239 / 0	0/0
HOR	IZONTAL REA	ACTIONS					
12		0/0	0/0	0/0	144 / -118	0/0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 12

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.83 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (11)

СН	ORDS					W E	BS		
MAX	K. FACTORED	FACTO	RED				MAX. FACTO	DRED	
ИЕМВ.	FORCE	VERT. LO	AD LC	1 MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PL	.F) (	CSI (LC)	<b>UNBRAC</b>	2	(LBS)	CSI (LC)	
R-TO		FROM	TO		LENGTH	FR-TO			
1-2	0 / 23	-77.3	-77.3	0.10(1)	10.00	11- 3	-83 / 159	0.03(7)	
2-3	-1587 / 693	-77.3	-77.3	0.52 (7)	4.83	3-10	-556 / 458	0.54(3)	
3- 4	-1142 / 576	-77.3	-77.3	0.50 (7)	5.51	10- 4	-290 / 649	0.26 (7)	
4- 5	-1142 / 584			0.47 (8)		10- 5	-469 / 411	0.43 (4)	
5-6	-1498 / 649			0.49 (8)		8- 5	-135 / 175	0.04(1)	
12-2	-1214 / 588	0.0	0.0	0.12(1)	7.31	2-11	-490 / 1445	0.32(1)	
7- 6	-1113 / 519	0.0	0.0	0.11 (1)	7.55	8- 6	-488 / 1370	0.31 (1)	
12-11	-183 / 156	-17.5	-17.5	0.15 (11	) 6.25				
11-10	-664 / 1471	-17.5	-17.5	0.30(1)	6.25				
10- 9	-459 / 1349	-17.5	-17.5	0.28 (1)	6.25				
9-8	-459 / 1349	-17.5	-17.5	0.28 (1)	6.25				
8- 7	-11 / 23	-17.5	-17.5	0.14 (11	) 6.25				

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

**RECEIVED** TOWN OF MILTON MAR 29, 2017 17-4978 **BUILDING DIVISION** 

# **DESIGN CRITERIA**

SPECIFIED LOADS: CH. PSF PSF

LL = DL = LL = DL = AD = 3.0 7.0 PSF TOTAL LOAD 33.3 PSF

### SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

TOTAL WEIGHT = 2 X 93 = 187 lb

THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012, BCBC 2012, ABC 2014

- CSA 086-09 - TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.80") CALCULATED VERT. DEFL.(LL) = L/999 (0.06") ALLOWABLE DEFL.(TL)= L/360 (0.80") CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.52 (2-3:7) , BC=0.30 (10-11:1) , WB=0.54 (3-10:3) , SSI=0.21 (2-3:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (8) (INPUT = 0.90 ) JSI METAL= 0.48 (6) (INPUT = 1.00 )



JOB NAME TRUSS NAME QUANTITY DRWG NO. TR-GREENPARK-LECCO RIDGE-BLOCK 327 Page 83 of 159 TW0317-048 TW0317-048 T25 Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:44 2017 Page Kott Lumber Uxbridge, Stouffville, ON, TW ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-Kp2Y7LeHTk586qXrTclRyJ19cXx8qk2cQ6CCGezcJJh 1-3-8 2.7.5 400 9-6-11 11-8-8 SCALE = 1:58.8 6 3x4 \\ 3x4 < 5x8 = 6.00 12 4x6 = 3 4x6 < 4x6 < 8 W10 412 W11 12 16 15 14 13 10 3x6 = 2x4 3x6 **4**x5 = 3x6 = 4x5 = 2x4 4x6 = 1·3·8 5·8 27-1-8 27-10-8 TOTAL WEIGHT = 2 X 110 = 221 LUMBER N. L. G. A. RULES CHORDS SIZE DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER **DESIGN CRITERIA** LUMBER DESCR BEARINGS FACTORED 3 DRY No.2 SPF SPF MAXIMUM FACTORED INPLIT REQRD SPECIFIED LOADS: No.2 GROSS REACTION GROSS REACTION TOP CH. LL =
DL =
LL =
DL =
AD = BRG LIPLIFT IN-SX 3.0 2x4 DRY No 2 VFRT HOR7 DOWN HORZ IN-SX PSF 8 2x4 No.2 16 5-8 5-8 HANGER BY OTHERS -673 16 -2x4 DRY No.2 SPF 1322 1347 -553 7.0 **PSF** No.2 MIN. SEAT SIZE: 3-8 TOTAL LOAD 33.3 16-12 No.2 PROVIDE ANCHORAGE AT BEARING JOINT 16 FOR 673 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 9 FOR 553 LBS FACTORED UPLIFT 9 No.2 SPACING = 24.0 IN. C/C ALL WEBS DRY SPF 2x3 No.2 ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, **EXCEPT** LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12 SHALL BE PROVIDED BY BUILDG. DESIGNER DRY: SEASONED LUMBER. PROVIDE FOR 201 LBS FACTORED HORIZONTAL REACTION AT JOINT 16 THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010 UNFACTORED REACTIONS MAX./MIN. COMPONENT REACTIONS SNOW LIVE 0/0 PLATES (table is in inches)
JT TYPE PLATES COMBINED DEAD SOIL THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09 W LEN Y Y X 2.00 2.75 TMVW-1 MT20 4.0 6.0 928 649 / 0 0/0 0/0 62 / -574 279 / 0 0/0 TTWW-m MT20 8.0 2.00 2.25 2.25 3.00 - TPIC 2011 HORIZONTAL REACTIONS 3.0 3.0 3.0 (55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED TMWW+t MT20 4.0 0/0 0/0 0/0 144 / -118 0/0 0/05.0 4.0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 16 TMWW-t MT20 1.50 1.75 ROOF LIVE LOAD TMVW-t BMV1+p 4.0 2.0 4.0 6.0 ALLOWABLE DEFL.(LL)= L/360 (0.93")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.93")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.27") MT20 4.0 <u>BRACING</u> TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.81 FT. BMWW-t MT20 5.0 2 00 1 50 BMWWW-4.0 MAX. UNBRACED BOTTOM CHORD LENGTH = 5.42 FT. OR RIGID CEILING DIRECTLY BS-t MT20 6.0 MT20 MT20 3.0 4.0 6.0 5.0 BMWW-t BMWW-t ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED. CSI: TC=0.50 (7-8:8), BC=0.56 (13-14:1), 2.00 1.50 BMWW-t MT20 3.0 6.0 1.50 2.00 WB=0.79 (5-11:3), SSI=0.20 (7-8:1) <u>LOADING</u> TOTAL LOAD CASES: (11) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

10 12

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED

CHORDS MAX. FACTORED WEBS MAX. FACTORED FACTORED MEMB. VERT. LOAD LC1 MAX MAX. FORCE MEMB. (LBS) (PLF) CSI (LC) UNBRAC (LBS) CSI (LC) FR-TO FROM ΤΌ LENGTH FR-TO 0/23 -77.3 -77.3 0.10 (1) 1- 2 2- 3 3- 4 4- 5 10.00 -396 / 224 0.06 (1) 15- 3 3-14 -1697 / 782 -77.3 -77.3 0.19 (7) -77.3 0.34 (7) 5.08 -757 / 1828 -841 / 438 0.40 (1) -77.3 -77.3 -3096 / 1394 0.13 (1) 3.81 -77.3 0.40 (7) -2274 / 1022 4.33 4-13 -1195 / 619 0.55(1)5-6 6-7 7-8 -1497 / 727 -1508 / 745 -77.3 -77.3 -77.3 0.36 (7) -77.3 0.49 (8) 5.15 4.99 13- 5 5-11 -199 / 589 -1025 / 640 11- 6 11- 7 10- 7 -1808 / 750 -77.3 -77 3 0 50 (8) 4 61 -468 / 1011 0 42 (7) 16- 2 -1447 / 687 0.39 (4) 9-8 -1303 / 581 0.0 0.0 0.13 (1) 7.12 -186 / 193 0.05 (1) 2-15 10- 8 -631 / 1581 -580 / 1659 0.35 (1) 0.37 (1) -17.5 -17.5 6.25 16-15 -183 / 156 0.04 (11) -17.5 -17.5 -17.5 -17.5 -17.5 -17.5 0.28 (1) -17.5 0.56 (1) -17.5 0.38 (1) 15-14 -770 / 1528 6.25 5.42 13-12 -889 / 2078 6.25 12-11 11-10 -889 / 2078 -549 / 1633 -17.5 0.38 (1) -17.5 0.32 (1) 6.25 6.25 10-9 -11 / 23 -17.5 -17.5 0.14 (11)

TL. WISE THE OF 100083566 NCEOFON March 10, 2017

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

**RECEIVED** TOWN OF MILTON MAR 29, 2017 17-4978 **BUILDING DIVISION**  COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (13) (INPUT = 0.90 )

JSI METAL= 0.51 (12) (INPUT = 1.00)

TRUSS MANUFACTURING PLANT

NAIL VALUES

RESPONSIBLE FOR QUALITY CONTROL IN THE

618 354 1667 822 2284 1656

TRUSS NAME POBPES GREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME QUANTITY DRWG NO. Page 84 of 159 TRUSS DESC 2 TW0317-048 TW0317-048 T26 Kott Lumber Uxbridge, Stouffville, ON, TW Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:45 2017 Page 1 ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-p0cwLhfvE2D?k\_610JpgVXaKMwKDZEVIfmxmp4zcJJm 1-3-8 5-3-5 400 6-10-11 11-8-8 SCALE = 1:58.9 6 4x4 🥢 3x4 < 5x6 = 4x6 < 6.00 12 3 **T2** 4x6 < 1-4-12 W2 W10 12 13 10 15 3x6 = 2x4 || 3x10 = 4x8 4x5 = 2x4 4x6 = 1·3·8 5·8 27.3-8 18 27-10-8 TOTAL WEIGHT = 2 X 112 = 225 lb

LUMBER				
N. L. G. A. R	ULES			
CHORDS	SIZE		LUMBER	DESCR.
1 - 3	2x4	DRY	No.2	SPF
3 - 4	2x4	DRY	No.2	SPF
4 - 6	2x4	DRY	No.2	SPF
6 - 8	2x4	DRY	No.2	SPF
15 - 2	2x4	DRY	No.2	SPF
9 - 8	2x4	DRY	No.2	SPF
15 - 12	2x4	DRY	No.2	SPF
12 - 9	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)										
JT	TYPE	PLATES	W	LEN	Υ	X				
2	TMVW-t	MT20	4.0	6.0	2.00	3.00				
3	TTWW-m	MT20	5.0	6.0	2.25	1.50				
4	TTW-h	MT20	4.0	6.0	2.25	3.25				
5	TMWW-t	MT20	4.0	4.0	2.00	1.25				
6	TTW+p	MT20	3.0	5.0	2.75	1.50				
7	TMWW-t	MT20	3.0	4.0	1.50	1.75				
8	TMVW-t	MT20	4.0	6.0		Edge				
9	BMV1+p	MT20	2.0	4.0						
10	BMWW-t	MT20	4.0	5.0	2.00	1.50				
11	BMWWW-t	MT20	4.0	6.0	1.75	3.00				
12	BS-t	MT20	3.0	6.0						
13	BMWWW-t	MT20	4.0	8.0	1.75	4.00				
14	BMWW-t	MT20	3.0	10.0	1.50	3.75				
15	BMV1+p	MT20	2.0	4.0						

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS	AND LOADINGS	SPECIFIED B	Y FABRICATOR	TO BE VERIFIED	) B
BUILDING DESIGNER					
DEADINGS					

DEA	KINGS						
	FACTO	RED	MAXIMU	M FACT	INPUT	REQRD	
	GROSS RE	EACTION	GROSS	REACTIO	N	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
15	1427	0	1461	201	-673	5-8	5-8
9	1322	0	1347	0	-553	HANGER E	BY OTHERS
						MIN. SEAT	SIZE: 1-8

PROVIDE ANCHORAGE AT BEARING JOINT 15 FOR 673 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 9 FOR 553 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 201 LBS FACTORED HORIZONTAL REACTION AT JOINT 15

	1ST LCASE	MAX./	MIN. COMPOR						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
15	999	712/0	0/0	0/0	85 / -665	287 / 0	0/0		
9	928	649 / 0	0/0	0/0	62 / -574	279 / 0	0/0		
HORIZONTAL REACTIONS									
15		0/0	0/0	0/0	144 / -118	0/0	0 /0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 15

<u>BRACING</u> TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.13 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (11)

СН	ORDS		WEBS						
MAX	K. FACTORED	FACTOR	ED				MAX. FACTO	RED	
MEMB.	FORCE	VERT. LOA	AD LC	1 MAX	MAX.	MEMB	FORCE	MAX	
	(LBS)	(PLF	=) (	CSI (LC)	<b>UNBRAC</b>		(LBS)	CSI (LC)	
FR-TO		FROM 1	ľΟ		LENGTH	FR-TO			
1-2	0 / 23	-77.3	-77.3	0.10(1)	10.00	14- 3	-176 / 154	0.04(1)	
2-3	-1871 / 838	-77.3	-77.3	0.45 (7)	4.64	3-13	-358 / 954	0.21 (1)	
3-4	-2381 / 1101	-77.3	-77.3	0.30 (7)	4.32	13-4	-1478 / 788	0.33 (1)	
4- 5	-2697 / 1306	-77.3	-77.3	0.32 (7)	4.13	13- 5	-571 / 1117	0.40 (7)	
5-6	-1498 / 744	-77.3	-77.3	0.24 (7)	5.27	5-11	-872 / 591	0.59(3)	
6- 7	-1513 / 742	-77.3	-77.3	0.49 (8)	4.98	11-6	-506 / 1058	0.45 (7)	
7-8	-1806 / 751	-77.3	-77.3	0.50 (8)	4.61	11- 7	-409 / 397	0.38 (4)	
15- 2	-1420 / 701	0.0	0.0	0.14(1)	6.89	10- 7	-194 / 188	0.05(1)	
9-8	-1301 / 582	0.0	0.0	0.13(1)	7.12	2-14	-614 / 1683	0.37 (1)	
						10-8	-581 / 1657	0.37 (1)	
15-14	-183 / 156	-17.5	-17.5	0.11 (11	) 6.25				
14-13	-781 / 1696	-17.5	-17.5	0.30(1)	6.25				
13-12	-749 / 1820	-17.5	-17.5	0.38 (1)	6.25				
12-11	-749 / 1820	-17.5	-17.5	0.38 (1)	6.25				
11-10	-550 / 1631	-17.5	-17.5	0.36(1)	6.25				
10-9	-11 / 23	-17.5	-17.5	0.13 (11	) 6.25				

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT (40-0-0) FIN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK
COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL
WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

**RECEIVED** TOWN OF MILTON MAR 29, 2017 17-4978 **BUILDING DIVISION** 

### **DESIGN CRITERIA**

SPECIFIED LOADS: TOP CH. LL = LL = 23.3 DL = 3.0 LL = 0.0 DL = 7.0 AD = 33.3 PSF TOTAL LOAD

### SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09 - TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.93") CALCULATED VERT. DEFL.(LL)= L/ 999 (0.13") ALLOWABLE DEFL.(TL)= L/360 (0.93") CALCULATED VERT. DEFL.(TL)= L/ 999 (0.25")

CSI: TC=0.50 (7-8:8) , BC=0.38 (11-13:1) , WB=0.59 (5-11:3) , SSI=0.20 (7-8:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

### NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (14) (INPUT = 0.90 ) JSI METAL= 0.53 (4) (INPUT = 1.00 )



POBPES GREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY DRWG NO. Page 85 of 159 TRUSS DESC. TW0317-048 2 TW0317-048 **T27** Kott Lumber Uxbridge, Stouffville, ON, TW Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:45 2017 Page ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-p0cwLhfvE2D?k\_610JpgVXaKEwJ\_ZBYIfmxmp4zcJJm <u> 1-3-</u>8 7-11-5 1257 7-11-5 1-3-8 SCALE: 1/4"=1" 4x8 = 2x4 || 4x8 = 6.00 12 3x10 / 3x10 < 3 2x4 || 12 13 11 15 10 3x6 = 3x4 = 4x5 = 3x8 3x4 = 4x5 = 1-3-8 27.5-0 1-3-8 5-8 28-4-0 TOTAL WEIGHT = 2 X 110 = 221 LUMBER N. L. G. A. RULES CHORDS SIZE DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER **DESIGN CRITERIA** LUMBER DESCR BEARINGS FACTORED DRY No.2 SPF SPF MAXIMUM FACTORED GROSS REACTION INPLIT REQRD SPECIFIED LOADS: LL = DL = LL = DL = AD = TOP CH. No.2 GROSS REACTION HORZ UPLIFT IN-SX 3.0 PSF 2x4 DRY No 2 VFRT HOR7 DOWN IN-SX 15 -2 2x4 No.2 10-2x4 DRY No.2 SPF 10 1459 -732 5-8 5-8 7.0 12 TOTAL LOAD 33.3 PROVIDE ANCHORAGE AT BEARING JOINT 15 FOR 732 LBS FACTORED 10 12 -No.2 SPACING = 24.0 IN. C/C ALL WEBS EXCEPT NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12 DRY: SEASONED LUMBER. PROVIDE FOR 144 LBS FACTORED HORIZONTAL REACTION AT JOINT 15 THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF UNFACTORED REACTIONS PART 9, NBCC 2010 PLATES (table is in inches) PLATES MT20 W LEN Y THIS DESIGN COMPLIES WITH: 4.0 10.0 1.50 3.75 - PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09 TMWW-t MT20 3.0 1.75 3.75 - TPIC 2011

4.0 2.0 4.0 TTWW-m MT20 8.0 TMW+w MT20 4.0 8.0 1.75 3.75 10.0 1.50 3.75 TTWW-m MT20 TMWW-t

3.0 a+VMT MT20 4.0 BMVW1-t MT20 MT20 4.0 5.0 1.75 1.75 BMWW-t 4.0 3.0 BS-t MT20 6.0 BMWWW-BMWW-t MT20 3.0 4.0 BMVW1-t MT20 4.0 5.0 1.75 1.75

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH

T.L. WISE

100083566

WOE OF ON

March 10, 2017

	1ST LCASE	MAX./	MIN. COMPON	NENT REACTION	JNS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
15	1014	723 / 0	0/0	0/0	112 / -710	291 / 0	0/0		
10	1014	723 / 0	0/0	0/0	27 / -710	291 / 0	0/0		
HORIZONTAL REACTIONS									
15		0/0	0/0	0/0	103 / -103	0/0	0 /0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 15, 10

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.28 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (11)

	C H O R D S W E B S MAX. FACTORED FACTORED MAX. FACTORED							
MEMB.		VERT. LO						
	(LBS)	(PL	.F) (	CSI (LC)	UNBRA	С	(LBS)	CSI (LC)
FR-TO	` ,	FROM	ΤΌ	. ,	LENGTH	HFR-TO		, ,
1- 2	0 / 23	-77.3	-77.3	0.10(1)	10.00	3-14	-37 / 225	0.04 (7)
2-3	-9 / 118	-77.3	-77.3	0.18 (7)	10.00	14- 4	-20 / 171	0.05 (11)
3- 4	-1879 / 972	-77.3	-77.3	0.30(7)	4.82	4-13	-409 / 558	0.50 (8)
4- 5	-2114 / 1142	-77.3	-77.3	0.51 (7)	4.28	13- 5	-622 / 481	0.23 (3)
5-6	-2114 / 1142	-77.3	-77.3	0.51 (7)	4.28	13- 6	-409 / 586	0.50 (7)
6- 7	-1859 / 972	-77.3	-77.3	0.30(8)	4.83	11-6	-21 / 177	0.05 (11)
7-8	-9 / 118	-77.3	-77.3	0.19 (8)	10.00	11- 7	-48 / 225	0.04 (8)
8- 9	0 / 23	-77.3	-77.3	0.10(1)	10.00	15- 3	-2064 / 962	0.78 (1)
15- 2	-235 / 213	0.0	0.0	0.03(7)	7.81	7-10	-2046 / 962	0.78 (1)
10-8	-234 / 213	0.0	0.0	0.03 (8)	7.81			
15-14	-887 / 1712	-17.5	-17.5	0.40(1)	6.25			
14-13	-704 / 1686	-17.5		0.40(1)				
13-12	-601 / 1663	-17.5	-17.5	0.40(1)	6.25			
12-11	-601 / 1663	-17.5	-17.5	0.40(1)	6.25			
11-10	-766 / 1665	-17.5	-17.5	0.40 (1)	6.25			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

**RECEIVED** TOWN OF MILTON MAR 29, 2017 17-4978 **BUILDING DIVISION**  (55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED

ALLOWABLE DEFL.(LL)= L/360 (0.94")
CALCULATED VERT. DEFL.(LL)= L/999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.94")
CALCULATED VERT. DEFL.(TL)= L/999 (0.18")

CSI: TC=0.51 (4-5:7) , BC=0.40 (10-11:1) , WB=0.78 (3-15:1) , SSI=0.23 (4-5:1)

TRUSS MANUFACTURING PLANT

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.89 (6) (INPUT = 0.90)

JSI METAL= 0.50 (15) (INPUT = 1.00)

NAIL VALUES

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10 COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE

ROOF LIVE LOAD

POBPES GREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY PLY DRWG NO. Page 86 of 159 TRUSS DESC. 2 TW0317-048 TW0317-048 T28 Kott Lumber Uxbridge, Stouffville, ON, TW Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:46 2017 Page ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-HCAIY1gX?MLsM8gEa1Kv1k6VrKgslkpuuQhJLXzcJJl 10.7.5 7-1-7 10.7.5 1.38

		4x4 =			SCALE = 1:50
			3x4 = 4x4 =		
4x6 = 2	6.00 12 3x4 3 T1 W3 W	4 W5 W6	15 6 W6 W5	3x4 7 7 T1 W3	4x6 % 8 9 0 %-1
16 2x4	15	14	13 12 3x6 = 3x6 -	11	10 2x4
	3x10 =	<b>3</b> x6 =	3x6 — 3x6 =	3x10 =	
1-3-8 5-8		2	7-5-0		1-3-8 5-8
5-8					5-8
		2	8-4-0		

LUMBER								
ULES								
SIZE		LUMBER	DESCR.					
2x4	DRY	No.2	SPF					
2x4	DRY	No.2	SPF					
2x4	DRY	No.2	SPF					
2x4	DRY	No.2	SPF					
2x4	DRY	No.2	SPF					
2x4	DRY	No.2	SPF					
2x4	DRY	No.2	SPF					
2x3	DRY	No.2	SPF					
	SIZE 2x4 2x4 2x4 2x4 2x4 2x4 2x4 2x4	SIZE 2x4 DRY	SIZE LUMBER 2x4 DRY No.2					

DRY: SEASONED LUMBER.

PLATES (table is in inches)									
JT	TYPE	PLATES	W	LEN	Υ	Χ			
2	TMVW-t	MT20	4.0	6.0	2.00	2.75			
3	TMWW-t	MT20	3.0	4.0	1.50	1.75			
4	TTW-m	MT20	4.0	4.0					
5	TMWW-t	MT20	3.0	4.0					
6	TTW-m	MT20	4.0	4.0					
7	TMWW-t	MT20	3.0	4.0	1.50	1.75			
8	TMVW-t	MT20	4.0	6.0	2.00	2.75			
10	BMV1+p	MT20	2.0	4.0					
11	BMWW-t	MT20	3.0	10.0	1.50	3.00			
12	BMWWW-t	MT20	3.0	6.0					
13	BS-t	MT20	3.0	6.0					
14	BMWWW-t	MT20	3.0	6.0					
15	BMWW-t	MT20	3.0	10.0	1.50	3.00			
16	RMV/1+n	MT20	2.0	4 0					

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS BUILDING DESIGNER	AND LOADINGS SPEC	IFIED BY FAI	BRICATOR TO I	BE VERIFIED BY
BEARINGS				
FACTORED	MAXIMUM FACTORED	INPUT	REQRD	
00000 DELOTION	0 D 0 0 0 D E 1 0 T 1 0 1 1			

	FACTORED		MAXIMU	M FACT	INPUT	REQRD	
	GROSS RI	EACTION	GROSS	REACTIO	BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
16	1449	0	1487	-170	-703	5-8	5-8
10	1449	0	1467	0	-703	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 16 FOR 703 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 10 FOR 703 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES,
SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 170 LBS FACTORED HORIZONTAL REACTION AT JOINT 16

UNFACTORED REACTIONS										
	1ST LCASE	SE MAX./MIN. COMPONENT REACTIONS								
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL			
16	1014	723 / 0	0/0	0/0	96 / -689	291 / 0	0/0			
10	1014	723 / 0	0/0	0/0	47 / -689	291 / 0	0/0			
16	IZONTAL REA	0/0	0 / 0 SPF NO.2 OR E	0 / 0 BETTER AT JO	121 / -121 DINT(S) 16, 1	0/0	0 /0			

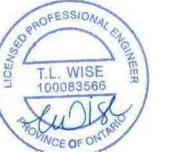
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.56 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (11)

СН	CHORDS WEBS									
MAX	K. FACTORED	FACTOR	RED				MAX. FACTO	DRED		
MEMB.	FORCE	VERT. LO	AD LC	1 MAX	MAX.	MEMB.	FORCE	MAX		
	(LBS)	(PL	F) (	CSI (LC)	UNBRAC		(LBS)	CSI (LC)		
FR-TO		FROM	TO		LENGTH	FR-TO				
1- 2	0 / 23	-77.3	-77.3	0.10(1)	10.00	15-3	-189 / 190	0.04(1)		
2-3	-1972 / 942	-77.3	-77.3	0.45 (7)	4.56	3-14	-362 / 345	0.26 (3)		
3- 4	-1694 / 870	-77.3	-77.3	0.44 (7)	4.86	14- 4	-206 / 473	0.14 (8)		
4- 5	-1507 / 849	-77.3	-77.3	0.23 (7)	5.28	14- 5	-201 / 235	0.19 (4)		
5-6	-1490 / 849	-77.3	-77.3	0.23 (8)	5.30	5-12	-215 / 235	0.20(3)		
6- 7	-1681 / 870	-77.3	-77.3	0.44 (8)	4.86	12-6	-205 / 478	0.14 (7)		
7-8	-1938 / 942	-77.3	-77.3	0.45 (8)	4.56	12-7	-367 / 345	0.26 (4)		
8- 9	0 / 23	-77.3	-77.3	0.10(1)	10.00	11- 7	-186 / 190	0.04(1)		
16- 2	-1445 / 728	0.0	0.0	0.14(1)	6.85	2-15	-727 / 1795	0.39(1)		
10-8	-1425 / 728	0.0	0.0	0.14 (1)	6.87	11-8	-727 / 1781	0.39 (1)		
40.45	450 / 474	47.5	47.5	0.40 (44)						
16-15	-152 / 174			0.10 (11)						
15-14	-858 / 1814			0.36 (1)						
14-13	-612 / 1609			0.33 (1)						
13-12	-612 / 1609			0.33 (1)						
12-11	-702 / 1760	-17.5	-17.5	0.36(1)	6.25					
11-10	-9 / 18	-17.5	-17.5	0.10 (11)	10.00					

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT (40-0-0) F-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK
COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL
WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON
(OPEN TERRAIN). AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY



March 10, 2017

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

**RECEIVED** TOWN OF MILTON MAR 29, 2017 17-4978 **BUILDING DIVISION** 

# **DESIGN CRITERIA**

SPECIFIED LOADS: 
 SPECIFIED LOADS:

 TOP
 CH.
 LL
 =
 23.3

 DL
 =
 3.0

 BOT
 CH.
 LL
 =
 0.0

 DL
 =
 7.0

 TOTAL
 LOAD
 =
 33.3
 PSF PSF

### SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

TOTAL WEIGHT = 2 X 117 = 234

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014

- CSA 086-09 - TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.94") CALCULATED VERT. DEFL.(LL)= L/ 999 (0.08") ALLOWABLE DEFL.(TL)= L/360 (0.94") CALCULATED VERT. DEFL.(TL)= L/ 999 (0.17")

CSI: TC=0.45 (7-8:8), BC=0.36 (14-15:1), WB=0.39 (2-15:1), SSI=0.18 (2-3:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

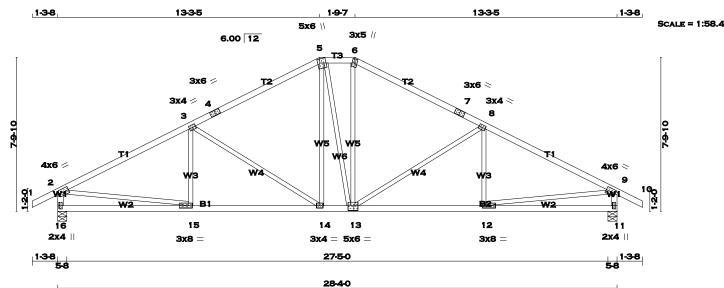
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (11) (INPUT = 0.90) JSI METAL= 0.53 (8) (INPUT = 1.00)

POBPES GREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY DRWG NO. Page 87 of 159 TRUSS DESC 2 TW0317-048 TW0317-048 T29 Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:46 2017 Page 1 Kott Lumber Uxbridge, Stouffville, ON, TW ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-HCAIY1gX?MLsM8gEa1Kv1k6T0KgaleuuuQhJLXzcJJl

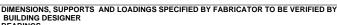


LUMBER				
N. L. G. A. R	ULES			
CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 5	2x4	DRY	No.2	SPF
5 - 6	2x4	DRY	No.2	SPF
6 - 7	2x4	DRY	No.2	SPF
7 - 10	2x4	DRY	No.2	SPF
16 - 2	2x4	DRY	No.2	SPF
11 - 9	2x4	DRY	No.2	SPF
16 - 13	2x4	DRY	No.2	SPF
13 - 11	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)										
JT	TYPE	PLATES	W	LEN	Υ	X				
2	TMVW-t	MT20	4.0	6.0	2.00	2.75				
3	TMWW-t	MT20	3.0	4.0	1.50	1.75				
4	TS-t	MT20	3.0	6.0						
5	TTWW+m	MT20	5.0	6.0	2.50	1.25				
6	TTW+m	MT20	3.0	5.0	2.50	1.25				
7	TS-t	MT20	3.0	6.0						
8	TMWW-t	MT20	3.0	4.0	1.50	1.75				
9	TMVW-t	MT20	4.0	6.0	2.00	2.75				
11	BMV1+p	MT20	2.0	4.0						
12	BMWW-t	MT20	3.0	8.0	1.50	2.25				
13	BSWWW-I	MT20	5.0	6.0	3.00	2.50				
14	BMWW-t	MT20	3.0	4.0						
15	BMWW-t	MT20	3.0	8.0	1.50	2.25				
16	DM///1 in	MTOO	2.0	4.0						

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.



BEARINGS										
	FACTORED		MAXIMU	M FACT	INPUT	REQRD				
	GROSS RE	EACTION	GROSS I	REACTIO	BRG	BRG				
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX			
16	1449	0	1479	-196	-665	5-8	5-8			
11	1449	0	1474	0	-665	5-8	5-8			

PROVIDE ANCHORAGE AT BEARING JOINT 16 FOR 665 LBS FACTORED PROVIDE ANCHORAGE AT BEARING JOINT 11 FOR 665 LBS FACTORED

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES

PROVIDE FOR 196 LBS FACTORED HORIZONTAL REACTION AT JOINT 16

UNFACTORED REACTIONS									
1ST LCASE MAX./MIN. COMPONENT REACTIONS									
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
16	1014	723 / 0	0/0	0/0	76 / -662	291 / 0	0/0		
11	1014	723 / 0	0/0	0/0	64 / -662	291 / 0	0/0		
16	IZONTAL REA	0/0	0 / 0 SPF NO.2 OR F	0 / 0 BETTER AT JO	140 / -140 DINT(S) 16, 1	0/0	0 /0		
	BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 16, 11								

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.29 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (11)

	O R D S	FACTOR	RFD			WE	B S MAX. FACTO	RED
MEMB.		VERT. LO						MAX
		(PL						
FR-TO	()	FROM					()	()
	0 / 23						-94 / 182	0.04 (7)
2-3	-1969 / 875							0.77 (3)
	-1467 / 728							0.24 (7)
	-1467 / 728			0.61 (7)			-110 / 103	0.13 (8)
	-1294 / 729			0.13 (8)			-182 / 388	0.20 (7)
	-1471 / 728		-77.3	0.61 (8)	4.87	13-8	-609 / 496	0.77 (4)
7-8	-1471 / 728	-77.3	-77.3	0.61 (8)	4.87	12-8	-96 / 184	0.04 (8)
8- 9	-1959 / 875	-77.3	-77.3	0.63 (8)	4.29	2-15	-646 / 1789	0.52 (7)
9-10	0 / 23	-77.3	-77.3	0.10(1)	10.00	12-9	-646 / 1782	0.52 (8)
16-2	-1430 / 697	0.0	0.0	0.14(1)	6.87			
11-9	-1425 / 696	0.0	0.0	0.14(1)	6.88			
16-15	-178 / 208	-17.5	-17.5	0.19 (11	) 6.25			
15-14	-816 / 1823	-17.5	-17.5	0.37 (1)	6.25			
14-13	-400 / 1313	-17.5	-17.5	0.27 (1)	6.25			
13-12	-624 / 1768	-17.5	-17.5	0.37 (1)	6.25			
12-11	-9 / 18	-17.5	-17.5	0.19 (11	) 10.00			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT (40-0-0) F-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK
COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL
WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON
(OPEN TERRAIN). AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY



March 10, 2017

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**RECEIVED** TOWN OF MILTON MAR 29, 2017 17-4978 **BUILDING DIVISION** 

### **DESIGN CRITERIA**

SPECIFIED LOADS: LL = 23.3 DL = 3.0 LL = 0.0 DL = 7.0 AD = 33.3 CH. PSF PSF TOTAL LOAD

### SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

TOTAL WEIGHT = 2 X 120 = 241 lb

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09 - TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.94") CALCULATED VERT. DEFL.(LL)= L/ 999 (0.09") ALLOWABLE DEFL.(TL)= L/360 (0.94") CALCULATED VERT. DEFL.(TL)= L/ 999 (0.17")

CSI: TC=0.63 (8-9:8), BC=0.37 (14-15:1), WB=0.77 (3-14:3), SSI=0.22 (2-3:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

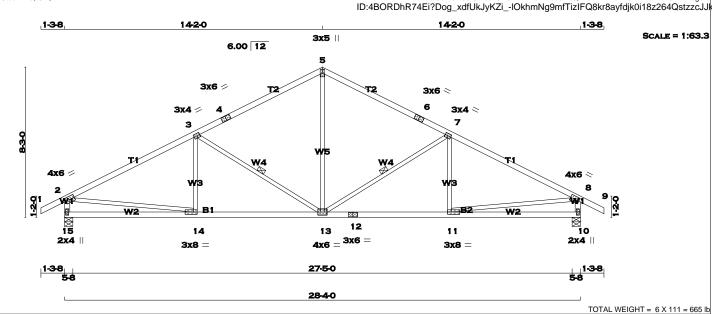
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (15) (INPUT = 0.90) JSI METAL= 0.53 (2) (INPUT = 1.00)

JOB PESC PREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY DRWG NO. Page 88 of 159 TRUSS DESC TW0317-048 TW0317-048 T30 6 Kott Lumber Uxbridge, Stouffville, ON, TW Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:47 2017 Page



LUMBER				
N. L. G. A. F	RULES			
CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 5	2x4	DRY	No.2	SPF
5 - 6	2x4	DRY	No.2	SPF
6 - 9	2x4	DRY	No.2	SPF
15 - 2	2x4	DRY	No.2	SPF
10 - 8	2x4	DRY	No.2	SPF
15 - 12	2x4	DRY	No.2	SPF
12 - 10	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)									
JΤ	TYPE	PLATES	W	LEN	Υ	Χ			
2	TMVW-t	MT20	4.0	6.0	2.00	2.75			
3	TMWW-t	MT20	3.0	4.0	1.50	1.75			
4	TS-t	MT20	3.0	6.0					
5	TTW+p	MT20	3.0	5.0					
6	TS-t	MT20	3.0	6.0					
7	TMWW-t	MT20	3.0	4.0	1.50	1.75			
8	TMVW-t	MT20	4.0	6.0	2.00	2.75			
10	BMV1+p	MT20	2.0	4.0					
11	BMWW-t	MT20	3.0	8.0	1.50	2.50			
12	BS-t	MT20	3.0	6.0					
13	BMWWW-t	MT20	4.0	6.0					
14	BMWW-t	MT20	3.0	8.0	1.50	2.50			
15	BMV1+p	MT20	2.0	4.0					

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

# DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

REAL	BEARINGS										
	FACTORED		MAXIMUM FACTORED			INPUT	REQRD				
	<b>GROSS RE</b>	ACTION	GROSS REACTION			BRG	BRG				
JT	VERT	HORZ	DOWN	HORZ	<b>UPLIFT</b>	IN-SX	IN-SX				
15	1449	0	1476	-204	-650	5-8	5-8				
10	1449	0	1476	0	-650	5-8	5-8				

PROVIDE ANCHORAGE AT BEARING JOINT 15 FOR 650 LBS FACTORED PROVIDE ANCHORAGE AT BEARING JOINT 10 FOR 650 LBS FACTORED

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES

PROVIDE FOR 204 LBS FACTORED HORIZONTAL REACTION AT JOINT 15

UNFACTORED REAC	TIONS
1ST LCASE	MAX./

	1ST LCASE	MAX./	<u>MIN. COMPON</u>	DNS			
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
15	1014	723 / 0	0/0	0/0	69 / -652	291 / 0	0/0
10	1014	723 / 0	0/0	0/0	69 / -652	291 / 0	0/0
HOR 15	IZONTAL RE	ACTIONS 0/0	0/0	0/0	146 / -146	0/0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 15, 10

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.17 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 7-13, 3-13. DBS = 20-0-0 . CBF = 79

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

WIN

(OPE

LOADING TOTAL LOAD CASES: (11)

СН	ORDS				WE	BS	
	K. FACTORED	FACTORED				MAX. FACTO	DRED
MEMB.	FORCE	VERT, LOAD LC	1 MAX	MAX.	MEMB.	FORCE	MAX
	(LBS)	(PLF)	CSI (LC)	UNBRAG	2	(LBS)	CSI (LC)
FR-TO	` ,	FROM TO	. ,	LENGTH	FR-TO	, ,	` '
1- 2	0 / 23	-77.3 -77.3	0.10(1)	10.00	13- 5	-352 / 814	0.46 (8)
2-3	-1956 / 845	-77.3 -77.3	0.70 (7)	4.17	13-7	-677 / 547	0.30 (4)
3- 4	-1405 / 699	-77.3 -77.3	0.67 (7)	4.84	11-7	-75 / 178	0.03 (8)
4- 5	-1405 / 699	-77.3 -77.3	0.67 (7)	4.84	3-13	-677 / 547	0.30 (3)
5- 6	-1405 / 699	-77.3 -77.3	0.67 (8)	4.84	14- 3	-75 / 178	0.03 (7)
6- 7	-1405 / 699				2-14	-613 / 1777	0.59 (7)
	-1956 / 846	-77.3 -77.3	0.70 (8)	4.17	11-8	-613 / 1777	0.59 (8)
8- 9	0 / 23		0.10(1)				
15- 2	-1423 / 685	0.0 0.0	0.14 (1)	6.88			
10-8	-1423 / 685	0.0 0.0	0.14 (1)	6.88			
15-14	-186 / 220		0.21 (11				
14-13	-795 / 1815		0.38 (1)				
13-12	-592 / 1762	<u>-17.5</u> -17.5	0.38 (1)	6.25			
12-11	591 / 11 2		(	6.25			
11-10	/ 1	17 - 5	0.2 11	) 10.00			
						. р	
WIND	APPELED	VEDC					
	)} FT-IN-SX REF	ERENCE HEIGH	T ABOVE	GRADE	AND US		
COE	READ ALL NO	TES ON THIS P	AGE AN	D ON TH	IF		FEM}.INTERNAL
WIN F	LAD ALL NO	. 20 0.1 111101	AUL AIN	2 24 11	-	C MAY BELL	ACTE ACTED 2011

IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED

IN THE DESIGN OF THIS COMPONENT.

ING SYSTEM) INTERNAL MAY BINAR ATO 2017 EAST (0-0) FT IN STAN ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE EAST (0-0) FT-IN-SX AWAY 17-4978

**BUILDING DIVISION** 

**DESIGN CRITERIA** 

SPECIFIED LOADS:

LL = 23.3 DL = 3.0 LL = 0.0 DL = 7.0 AD = 33.3 CH. PSF PSF TOTAL LOAD PSF

### SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014 - CSA 086-09 - TPIC 2011

ROOF LIVE LOAD

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED

ALLOWABLE DEFL.(LL)= L/360 (0.94") CALCULATED VERT. DEFL.(LL) = L/999 (0.09") ALLOWABLE DEFL.(TL)= L/360 (0.94") CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.70 (7-8:8) , BC=0.38 (13-14:1) , WB=0.59 (8-11:8) , SSI=0.24 (2-3:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (11) (INPUT = 0.90 ) JSI METAL= 0.53 (8) (INPUT = 1.00 )



JOB PESC TR-GREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY DRWG NO. Page 89 of 159 TW0317-048 TW0317-048 T31 16 Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:47 2017 Page 1 Kott Lumber Uxbridge, Stouffville, ON, TW

7-7-0 1-3-8 SCALE = 1:21.8 2x4 || л 3.50 12 3x4 = 3x4 = 9 曲 B 7 6 3x4 = **3**x4 = 1.3-8 7-10-8 8-5-8

LUMBER				
N. L. G. A. R	ULES			
CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
6 - 4	2x3	DRY	No.2	SPF
8 - 2	2x4	DRY	No.2	SPF
8 - 5	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
1				

DRY: SEASONED LUMBER.

### PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Υ	X	
2	TMVW-p	MT20	3.0	4.0	1.00	2.00	
3	TMWW-t	MT20	3.0	4.0			
4	TMV+p	MT20	2.0	4.0			
6	BMVW-t	MT20	3.0	4.0	1.50	1.75	
7	BMWW-t	MT20	3.0	4.0	1.50	1.75	
8	BMV1+p	MT20	2.0	4.0			

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

### DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS										
FACTORED		MAXIMUM FACTORED			INPUT	REQRD				
GROSS R	EACTION	CTION GROSS REACTION			BRG	BRG				
VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX				
501	0	501	203	-261	5-8	5-8				
337	0	337	0	-174	1-8	1-8				
	FACTOR GROSS RE VERT 501	FACTORED GROSS REACTION VERT HORZ 501 0	FACTORED MAXIMU GROSS REACTION GROSS I VERT HORZ DOWN 501 0 501	FACTORED MAXIMUM FACTOR GROSS REACTION GROSS REACTION VERT HORZ DOWN HORZ 501 0 501 203	FACTORED         MAXIMUM FACTORED           GROSS REACTION         GROSS REACTION           VERT         HORZ         DOWN         HORZ         UPLIFT           501         203         -261	FACTORED MAXIMUM FACTORED INPUT GROSS REACTION GROSS REACTION BRG VERT HORZ DOWN HORZ UPLIFT IN-SX 501 0 501 203 -261 5-8				

PROVIDE ANCHORAGE AT BEARING JOINT 8 FOR 261 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 5 FOR 174 LBS FACTORED UPLIFT

PROVIDE FOR 203 LBS FACTORED HORIZONTAL REACTION AT JOINT 8

### UNFACTORED REACTIONS

	1ST LCASE	MAX./	MIN. COMPO						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
8	349	257 / 0	0/0	0/0	0 / -246	92 / 0	0/0		
5	238	158 / 0	0/0	0/0	0 / -175	80 / 0	0/0		
HO	HORIZONTAL REACTIONS								
8		0/0	0/0	0/0	145 / 0	0/0	0 /0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 8, 5

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (11)

CHC	CHORDS					WEBS			
MAX.	FACTORED	FACTO	RED				MAX. FACTO	ORED	
MEMB.	FORCE	VERT. LC	AD LC	1 MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PL	_F) (	CSI (LC)	UNBRAC		(LBS)	CSI (LC)	
FR-TO		FROM	TO		LENGTH	FR-TO			
1- 2	0 / 14	-77.3	-77.3	0.10(1)	10.00	7- 3	0 / 97	0.03 (11)	
2-3	-630 / 326	-77.3	-77.3	0.18 (5)	6.25	3-6	-669 / 413	0.19(1)	
3- 4	-53 / 38	-77.3	-77.3	0.14 (5)	6.25	2-7	-272 / 626	0.14(1)	
6- 4	-115 / 84	0.0	0.0	0.12 (5)	7.81				
8- 2	-489 / 286	0.0	0.0	0.05 (1)	7.81				
0.7	400 / 0	47.5	47.5	0.44 (4)	0.05				
8- 7	-190 / 0			0.11 (1)	6.25				
7-6	-323 / 611			0.49 (1)					
6- 5	0/0	-17.5	-17.5	0.39(1)	10.00				

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CPCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

### **DESIGN CRITERIA**

ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-lOkhmNg9mfTizIFQ8kr8ayflrk\_?1FH264QstzzcJJk

SPECIFIED LOADS:							
TOP	CH.	LL	=	23.3	PS		
		DL	=	3.0	PS		
BOT	CH.	LL	=	0.0	PS		

SF SF  $\begin{array}{ccc} DL &=& 7.0 \\ TOTAL & LOAD &=& 33.3 \end{array}$ PSF PSF

### SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

TOTAL WEIGHT = 16 X 30 = 475 lb

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014 - CSA 086-09 TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.28") CALCULATED VERT. DEFL.(LL) = L/999 (0.06") ALLOWABLE DEFL.(TL)= L/360 (0.28") CALCULATED VERT. DEFL.(TL) = L/874 (0.12")

CSI: TC=0.18 (2-3:5), BC=0.49 (6-7:1), WB=0.19 (3-6:1) , SSI=0.26 (5-6:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

## NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (7) (INPUT = 0.90 ) JSI METAL= 0.23 (7) (INPUT = 1.00 )

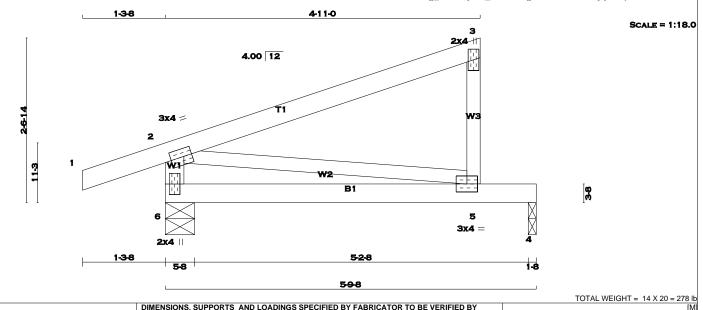




READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

JOB NAME TRUSS NAME QUANTITY PLY DRWG NO. JOBPESC TR-GREENPARK-LECCO RIDGE-BLOCK 327 Page 90 of 159 TW0317-048 TW0317-048 T32 14

Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:47 2017 Page 1 Kott Lumber Uxbridge, Stouffville, ON, TW ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-lOkhmNg9mfTizIFQ8kr8ayfjhk2q1Ho264QstzzcJJk



LUMBER				
N. L. G. A. I	RULES			
CHORDS	SIZE		LUMBER	DESCR.
1 - 3	2x4	DRY	No.2	SPF
5 - 3	2x3	DRY	No.2	SPF
6 - 2	2x4	DRY	No.2	SPF
6 - 4	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEAS	ONED L	UMBER.		

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Υ	X	
2	TMVW-t	MT20	3.0	4.0	1.50	1.50	
3	TMV+p	MT20	2.0	4.0			
5	BMVW-t	MT20	3.0	4.0			
6	BMV1+p	MT20	2.0	4.0			

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

### DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

EQRD
RG
N-SX
-8
-8
F

PROVIDE ANCHORAGE AT BEARING JOINT 6 FOR 194 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 4 FOR 150 LBS FACTORED UPLIFT

PROVIDE FOR 175 LBS FACTORED HORIZONTAL REACTION AT JOINT 6

### UNFACTORED REACTIONS

	1ST LCASE	MAX./	MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
6	260	194 / 0	0/0	0/0	0 / -181	66 / 0	0/0		
4	150	97 / 0	0/0	0/0	0 / -112	53 / 0	0/0		
HOR 6	IZONTAL RE	ACTIONS 0/0	0/0	0/0	125 / 0	0/0	0 /0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 6, 4

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (11)

CHC	CHORDS					WEBS				
MAX.	FACTORED	FACTOR	RED				MAX. FACTO	RED		
MEMB.	FORCE	VERT. LO	AD LC1	1 MAX	MAX.	MEMB.	FORCE	MAX		
	(LBS)	(PL	F) (	CSI (LC)	UNBRAC	)	(LBS)	CSI (LC)		
FR-TO		FROM	TO		LENGTH	FR-TO				
1- 2	0 / 16	-77.3	-77.3	0.10(1)	10.00	2- 5	0 / 136	0.03 (6)		
2-3	-69 / 11	-77.3	-77.3	0.32(1)	6.25					
5-3	-190 / 158	0.0	0.0	0.10 (5)	7.81					
6- 2	-294 / 206	0.0	0.0	0.03 (7)	7.81					
6- 5	-161 / 0	-17.5	-17.5	0.25 (1)	6.25					
5- 4	0/0	-17.5	-17.5	0.24(1)	10.00					

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT 440-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK
COEFFICIENTS, CPCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL
WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.

### **DESIGN CRITERIA**

SPECIFIED LOADS: LL = 23.3 DL = 3.0 LL = 0.0 DL = 7.0 AD = 33.3 CH. PSF

PSF TOTAL LOAD PSF

### SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014 - CSA 086-09 TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.19") CALCULATED VERT. DEFL.(TL) = L/586 (0.12")

CSI: TC=0.32 (2-3:1) , BC=0.25 (5-6:1) , WB=0.03 (2-5:6) , SSI=0.17 (4-5:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

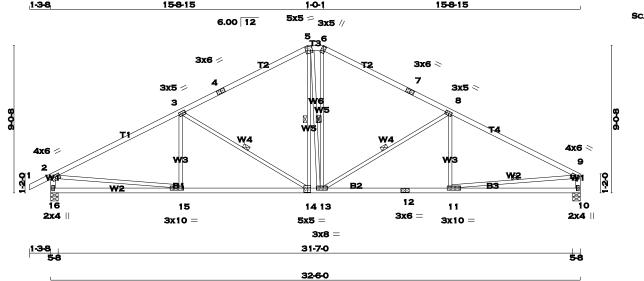
JSI GRIP= 0.29 (2) (INPUT = 0.90 ) JSI METAL= 0.08 (6) (INPUT = 1.00 )





READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

JOB PESC PREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY DRWG NO. Page 91 of 159 TRUSS DESC TW0317-048 TW0317-048 T33 Kott Lumber Uxbridge, Stouffville, ON, TW Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:48 2017 Page ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-DbH3zjhoXzbZbSqciSNN79Clu8JZmVMBLkAQPPzcJJj 15-8-15 1-3-8 15-8-15 5x5 = <sub>3x5 //</sub> SCALE = 1:70.7 6.00 12 3x6 3x6 <



LUMBER				
N. L. G. A. R	ULES			
CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 5	2x4	DRY	No.2	SPF
5 - 6	2x4	DRY	No.2	SPF
6 - 7	2x4	DRY	No.2	SPF
7 - 9	2x4	DRY	No.2	SPF
16 - 2	2x4	DRY	No.2	SPF
10 - 9	2x4	DRY	No.2	SPF
16 - 14	2x4	DRY	No.2	SPF
14 - 12	2x4	DRY	No.2	SPF
12 - 10	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER

PL/	PLATES (table is in inches)								
JT	TYPE	PLATES	W	LEN	Υ	Χ			
2	TMVW-t	MT20	4.0	6.0	1.50	2.75			
3	TMWW-t	MT20	3.0	5.0					
4	TS-t	MT20	3.0	6.0					
5	TTWW-m	MT20	5.0	5.0	2.50	1.25			
6	TTW+h	MT20	3.0	5.0	2.75	1.00			
7	TS-t	MT20	3.0	6.0					
8	TMWW-t	MT20	3.0	5.0					
9	TMVW-t	MT20	4.0	6.0	1.50	2.75			
10	BMV1+p	MT20	2.0	4.0					
11	BMWW-t	MT20	3.0	10.0	1.50	3.50			
12	BS-t	MT20	3.0	6.0					
13	BMWWW-t	MT20	3.0	8.0					
14	BSWW-I	MT20	5.0	5.0	3.25	2.50			
15	BMWW-t	MT20	3.0	10.0	1.50	3.50			
16	BMV1+p	MT20	2.0	4.0					

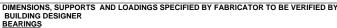
A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH

T.L. WISE

100083566

NCE OF ON

March 10, 2017



TOTAL WEIGHT = 137 lb

PSF 3.0 PSF

7.0 PSF

IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL

- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED

ALLOWABLE DEFL.(LL)= L/360 (1.08")
CALCULATED VERT. DEFL.(LL)= L/999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (1.08")
CALCULATED VERT. DEFL.(TL)= L/999 (0.25")

CSI: TC=0.86 (2-3:1), BC=0.47 (14-15:1),

WB=0.97 (2-15:7), SSI=0.27 (2-3:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.89 (14) (INPUT = 0.90)

JSI METAL= 0.62 (2) (INPUT = 1.00)

OR SMALL BUILDING REQUIREMENTS OF

33.3 PSF

**DESIGN CRITERIA** SPECIFIED LOADS:

CH.

TOTAL LOAD

PART 9, NBCC 2010 THIS DESIGN COMPLIES WITH:

- CSA 086-09 - TPIC 2011

NAIL VALUES

EAST (0-0) FT-IN-SX AWAY
BUILDING DIVISION

ROOF LIVE LOAD

SPACING =

LL = DL = LL = DL = AD =

24.0

	FACTORED		MAXIMUM FACTORED			INPUT	REQRD				
	GROSS RE	EACTION	GROSS I	REACTIC	BRG	BRG					
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX				
16	1646	0	1680	242	-745	5-8	5-8				
10	1541	0	1576	0	-683	5-8	5-8				

PROVIDE ANCHORAGE AT BEARING JOINT 16 FOR 745 LBS FACTORED PROVIDE ANCHORAGE AT BEARING JOINT 10 FOR 683 LBS FACTORED

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES

PROVIDE FOR 242 LBS FACTORED HORIZONTAL REACTION AT JOINT 16

### UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
16	1153	820 / 0	0/0	0/0	85 / -747	333 / 0	0/0		
10	1082	757 / 0	0/0	0/0	87 / -697	325 / 0	0/0		
HOR	IZONTAL RE	ACTIONS							
16		0/0	0/0	0/0	173 / -143	0/0	0 /0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 16, 10

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.56 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

- 1 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 3-14, 5-14, 5-13, 6-13, 8-13. DBS =
- 1 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 9-11. DBS = 14-0-0 . CBF = 90 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

WIN {OPE

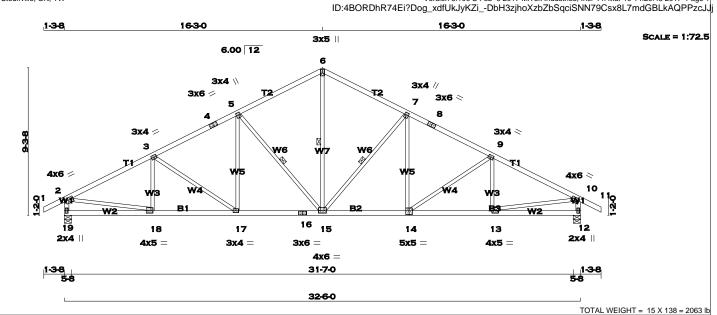
LOADING TOTAL LOAD CASES: (11)

ı										
	СН	ORDS					WE			
	MAX	(. FACTORED	FACTOR	RED				MAX. FACTO	ORED	
ı	MEMB.	FORCE	VERT. LO.	AD LC1	1 MAX	MAX.	MEMB.	FORCE	MAX	
		(LBS)	(PL	F) (	CSI (LC)	UNBRAG	0	(LBS)	CSI (LC)	
	FR-TO		FROM	TO		LENGTH	FR-TO			
	1- 2	0 / 23	-77.3	-77.3	0.10(1)	10.00	15-3	-67 / 196	0.04 (11)	
1	2-3	-2315 / 1009	-77.3	-77.3	0.86(1)	3.56	3-14	-777 / 615	0.45 (3)	
	3- 4	-1667 / 821	-77.3	-77.3	0.83 (7)	4.21	14- 5	-250 / 440	0.13 (7)	
	4- 5	-1667 / 821	-77.3	-77.3	0.83 (7)	4.21	5-13	-128 / 107	0.07 (8)	
	5- 6	-1468 / 811	-77.3	-77.3	0.12 (8)	5.43	13-6	-232 / 472	0.12 (7)	
	6- 7	-1671 / 822	-77.3	-77.3	0.83 (8)	4.21	13-8	-775 / 611	0.45 (4)	
	7-8	-1671 / 822	-77.3	-77.3	0.83 (8)	4.21	11-8	-72 / 202	0.04 (8)	
	8- 9	-2308 / 1004	-77.3	-77.3	0.86(1)	3.56	2-15	-750 / 2100	0.97 (7)	
	16- 2	-1623 / 784	0.0	0.0	0.16(1)	6.54	11-9	-781 / 2092	0.46 (1)	
	10-9	-1518 / 721	0.0	0.0	0.15 (1)	6.71				
	16-15	-225 / 205	-17.5	-17.5	0.27 (11	) 6.25				
	15-14	<u>-9</u> 70/2 <u>138</u>	-17.5	-17.5	0.47 (1)	6.25				
	14-13	44' / 14			(	6.25				
	13-12	/2 /2	17	- 5 - 5	0.4 1)	6.25				
	12-11	/ 2	17	- 5	0.4 1)	6.25				
	11-10	/ 18	17	- 5	0.2 11	) 10.00		D	ECEN/ED	
								K	ECEIVED	
	_							_ TOW	'N OF MILTON	1
	WINE	READ ALL NO	TES ON T	HIS PA	GE ANI	ON TH	IF		OF { 9.0} PSF AT	
	{+U-U	NGINEERING							NRPE29K, 2017	
	COL							0 1140 0101	EM}.INTERNAL	
		S AN INTEGR							.do/atteb/don	
	{OPF (	CONTAINS SP	ECIFICAT	IONS A	AND CR	ITERIA I	JSED	EAST (0-0)	FT-IN-SX AWAY	

IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED

IN THE DESIGN OF THIS COMPONENT.

JOB PESC PREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY DRWG NO. Page 92 of 159 TRUSS DESC TW0317-048 TW0317-048 T34 15 Kott Lumber Uxbridge, Stouffville, ON, TW Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:48 2017 Page

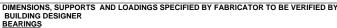


<u>LUMBER</u>				
N. L. G. A. R	ULES			
CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 6	2x4	DRY	No.2	SPF
6 - 8	2x4	DRY	No.2	SPF
8 - 11	2x4	DRY	No.2	SPF
19 - 2	2x4	DRY	No.2	SPF
12 - 10	2x4	DRY	No.2	SPF
19 - 16	2x4	DRY	No.2	SPF
16 - 14	2x4	DRY	No.2	SPF
14 - 12	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
FXCEPT				

DRY: SEASONED LUMBER.

<u>PL/</u>	PLATES (table is in inches)									
JT	TYPE	PLATES	W	LEN	Υ	X				
2	TMVW-t	MT20	4.0	6.0	1.50	2.75				
3	TMWW-t	MT20	3.0	4.0	1.50	1.75				
4	TS-t	MT20	3.0	6.0						
5	TMWW+t	MT20	3.0	4.0	1.75	0.75				
6	TTW+p	MT20	3.0	5.0	2.75	1.50				
7	TMWW+t	MT20	3.0	4.0	1.75	0.75				
8	TS-t	MT20	3.0	6.0						
9	TMWW-t	MT20	3.0	4.0	1.50	1.75				
10	TMVW-t	MT20	4.0	6.0	1.50	2.75				
12	BMV1+p	MT20	2.0	4.0						
13	BMWW-t	MT20	4.0	5.0	1.75	1.50				
14	BSWW-I	MT20	5.0	5.0	3.00	2.50				
15	BMWWW-t	MT20	4.0	6.0	1.75	3.00				
16	BS-t	MT20	3.0	6.0						
17	BMWW-t	MT20	3.0	4.0						
18	BMWW-t	MT20	4.0	5.0	1.75	1.50				
19	BMV1+p	MT20	2.0	4.0						

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.



	FACTOR	RED	MAXIMUM FACTORED GROSS REACTION			INPUT	REQRD			
	<b>GROSS RE</b>	ACTION				BRG	BRG			
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX			
19	1646	0	1678	230	-738	5-8	5-8			
12	1646	0	1678	0	-738	5-8	5-8			

PROVIDE ANCHORAGE AT BEARING JOINT 19 FOR 738 LBS FACTORED PROVIDE ANCHORAGE AT BEARING JOINT 12 FOR 738 LBS FACTORED

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES

PROVIDE FOR 230 LBS FACTORED HORIZONTAL REACTION AT JOINT 19

UNFACTORED REACT	TIONS
40T1040E	1111

	1ST LCASE	MAX./	<u>'MIN. COMPON</u>	<u>IENT REACTIO</u>	DNS		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
19	1153	820 / 0	0/0	0/0	81 / -741	333 / 0	0/0
12	1153	820 / 0	0/0	0/0	81 / -741	333 / 0	0/0
HOR 19	IZONTAL RE	ACTIONS 0/0	0/0	0/0	165 / -165	0/0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 19, 12

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 6-15, 7-15, 5-15. DBS = 20-0-0. CBF =

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING TOTAL LOAD CASES: (11)

	СН	ORDS					WE	BS			
ı	MAX	. FACTORED	FACTO	RED				MAX	. FACT	ORED	
ı	MEMB.	FORCE	VERT. LO	AD LC	1 MAX	MAX.	MEMB.		FORCE	MAX	
ı		(LBS)	(PL	_F) (	CSI (LC)	<b>UNBRAC</b>	)	(	LBS)	CSI (LC)	
ı	FR-TO		FROM	TO		LENGTH	FR-TO				
ı	1- 2	0 / 23	-77.3	-77.3	0.10(1)	10.00	15-6	-515	/ 1082	0.28 (8)	
	2-3	-2290 / 990	-77.3	-77.3	0.41 (7)	4.25	15- 7	-673	/ 518	0.30 (4)	
i	3- 4	-2060 / 952	-77.3	-77.3	0.41 (7)	4.50	14- 7	-98	/ 249	0.07 (8)	
	4- 5	-2060 / 952	-77.3	-77.3	0.41 (7)	4.50	14- 9	-282	/ 287	0.21 (4)	
	5-6	-1611 / 830	-77.3	-77.3	0.37 (7)	4.96	13- 9	-216	/ 198	0.05 (1)	
	6- 7	-1611 / 830	-77.3	-77.3	0.37 (8)	4.96	5-15	-673	/ 518	0.30 (3)	
	7-8	-2060 / 952	-77.3	-77.3	0.41 (8)	4.50	17- 5	-97	/ 249	0.07 (7)	
	8- 9	-2060 / 952	-77.3	-77.3	0.41 (8)	4.50	3-17	-282	/ 287	0.21 (3)	
	9-10	-2290 / 990	-77.3	-77.3	0.41 (8)	4.25	18- 3	-216	/ 198	0.05 (1)	
	10-11	0 / 23	-77.3	-77.3	0.10(1)	10.00	2-18	-764	/ 2078	0.46 (1)	
	19- 2	-1636 / 764	0.0	0.0	0.16(1)	6.52	13-10	-765	/ 2078	0.46 (1)	
	12-10	-1636 / 764	0.0	0.0	0.16(1)	6.52					
	19-18	-213 / 247			0.12 (11						
	18-17	-969 / 2110			0.37 (1)						
	17-16	-732 / 1877			0.35 (1)						
	16-15	-732 / 1877			0.35 (1)						
	15-14	-556 / 1839			0.35 (1)				П	ECEIVED.	
	14-13	-739 / 2046			0.37 (1)					ECEIVED	
	13-12	-9 / 18	-17.5	-17.5	0.12 (11	) 10.00			TOV	VN OF MILTOI	١
										IAD 00 0047	
									IVI	IAR 29, 2017	

17-4978 **BUILDING DIVISION** 

### **DESIGN CRITERIA**

SPECIFIED LOADS: LL = DL = LL = DL = AD = CH. 3.0 PSF 7.0 PSF TOTAL LOAD 33.3 PSF

### SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014

- CSA 086-09 TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.08") CALCULATED VERT. DEFL.(LL) = L/999 (0.11") ALLOWABLE DEFL.(TL)= L/360 (1.08") CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.41 (9-10:8) , BC=0.37 (13-14:1) , WB=0.46 (10-13:1) , SSI=0.18 (9-10:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

### NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (13) (INPUT = 0.90 ) JSI METAL= 0.62 (18) (INPUT = 1.00 )



TRUSS NAME
TO THE TRUSS NAME
TO THE TRUSS NAME
TO TRUSS DESC.

Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:48 2017 Page 2 ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-DbH3z|hoXzbZbSqciSNN79Csx8L7mdGBLkAQPPzcJJj

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.0) PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND PORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESCRIPTION (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

JOB PR-GREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY PLY DRWG NO. Page 94 of 159 TW0317-048 TW0317-048 T35 Kott Lumber Uxbridge, Stouffville, ON, TW Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:49 2017 Page

ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-hnrRA3iQIHkQDcPpF9ucfNk40YloVAHLaOvzyrzcJJi 2-10-4 2-10-4 1-3-8 1-3-8 428 SCALE: 1/2"=1 4x5 = 3x4 // 3 8.00 12 3x5 / 3x5 > 10-13 KVA 10-13 # # BI 9 8 3x4 = 3x6 = 2x4

LUMBER				
N. L. G. A. R				
CHORDS	SIZE		LUMBER	DESCR.
1 - 3	2x4	DRY	No.2	SPF
3 - 4	2x4	DRY	No.2	SPF
4 - 6	2x4	DRY	No.2	SPF
10 - 2	2x4	DRY	No.2	SPF
7 - 5	2x4	DRY	No.2	SPF
10 - 7	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
FXCEPT				-

1-3-8

5-8

DRY: SEASONED LUMBER.

PLATES (table is in inches)
JT TYPE PLATES LEN Y 5.0 5.0 4.0 1.50 2.00 1.75 1.50 TMVW-t MT20 TTWW-m TTW+m MT20 3.0 TMVW-t MT20 3.0 5.0 1.50 2.00 BMV1+p MT20 3.0 RMWWW-t MT20 6.0 BMWW-t 2.0 10 BMV1+p MT20 4.0

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TI 20 PLATES IS ALLOWED

DIMENSIONS, SUPPORTS	AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY
BUILDING DESIGNER	

900

9-11-0

BEAI	RINGS						
	FACTOR	ED	MAXIMUM FACTORED			INPUT	REQRD
	<b>GROSS RE</b>	ACTION	GROSS F	REACTIO	BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
10	576	0	611	-145	-293	5-8	5-8
7	576	0	599	0	-293	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 10 FOR 293 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 7 FOR 293 LBS FACTORED UPLIFT

PROVIDE FOR 145 LBS FACTORED HORIZONTAL REACTION AT JOINT 10

### UNFACTORED REACTIONS

	1ST LCASE	MAX./	MIN. COMPON	IENT REACTION	DNS		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
10	402	295 / 0	0/0	0/0	87 / -278	107 / 0	0/0
7	402	295 / 0	0/0	0/0	57 / -278	107 / 0	0/0
HOR 10	IZONTAL RE	ACTIONS 0/0	0/0	0/0	103 / -103	0/0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 10, 7

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (11)

	RDS				WEBS				
MAX.	FACTORED	FACTOR	RED				MAX. FACTO	DRED	
MEMB.	FORCE	VERT. LO	AD LC	1 MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PL	F) (	CSI (LC)	UNBRAC		(LBS)	CSI (LC)	
FR-TO		FROM	TO		LENGTH	FR-TO			
1- 2	0/29	-77.3	-77.3	0.10(1)	10.00	9-3	-13 / 71	0.02 (11)	
2- 3	-496 / 244	-77.3	-77.3	0.15 (7)	6.25	3-8	-55 / 59	0.02 (5)	
3- 4	-409 / 262	-77.3	-77.3	0.20 (8)	6.25	8- 4	-16 / 85	0.02 (11)	
4- 5	-494 / 245	-77.3	-77.3	0.15 (8)	6.25	2-9	-122 / 410	0.09(1)	
5- 6	0/29	-77.3	-77.3	0.10(1)	10.00	8- 5	-123 / 425	0.09 (1)	
10- 2	-590 / 305	0.0	0.0	0.06(1)	7.81				
7- 5	-578 / 306	0.0	0.0	0.06(1)	7.81				
10-9	-133 / 139	-17.5	-17.5	0.05 (11)	6.25				
9- 8	-138 / 430	-17.5	-17.5	0.09(1)	6.25				
8- 7	-5 / 11	-17.5	-17.5	0.05 (11)	10.00				

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.

## **DESIGN CRITERIA**

5-8

SPECIFIED LOADS: LL = DL = LL = DL = AD = CH. 3.0 PSF

1-3-8

7.0 PSF TOTAL LOAD 33.3

### SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

TOTAL WEIGHT = 40 lb

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09

- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.33")
CALCULATED VERT. DEFL.(LL)= L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.33")
CALCULATED VERT. DEFL.(TL)= L/999 (0.01")

CSI: TC=0.20 (3-4:8), BC=0.09 (8-9:1), WB=0.09 (5-8:1), SSI=0.13 (3-4:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (8) (INPUT = 0.90 ) JSI METAL= 0.18 (2) (INPUT = 1.00)

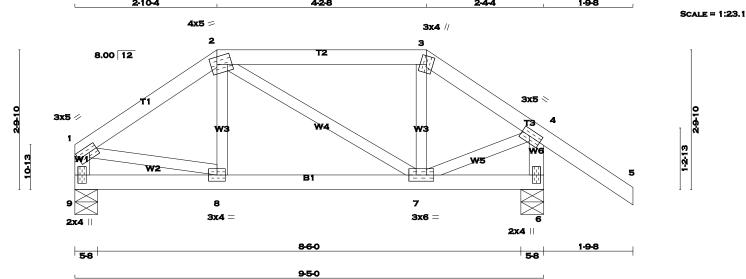




READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

JOB PR-GREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY PLY DRWG NO. Page 95 of 159 TW0317-048 TW0317-048 T36 Kott Lumber Uxbridge, Stouffville, ON, TW

Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:49 2017 Page ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-hnrRA3iQIHkQDcPpF9ucfNk48YltVASLaOvzyrzcJJi 2-10-4 198 4-2-8 4x5 = 3x4 //



LUMBER										
N. L. G. A. RULES										
CHORDS	SIZE		LUMBER	DESCR.						
1 - 2	2x4	DRY	No.2	SPF						
2 - 3	2x4	DRY	No.2	SPF						
3 - 5	2x4	DRY	No.2	SPF						
9 - 1	2x4	DRY	No.2	SPF						
6 - 4	2x4	DRY	No.2	SPF						
9 - 6	2x4	DRY	No.2	SPF						
ALL WEBS	2x3	DRY	No.2	SPF						
EXCEPT										

DRY: SEASONED LUMBER

### PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Υ	Χ
1	TMVW-t	MT20	3.0	5.0	1.50	2.00
2	TTWW-m	MT20	4.0	5.0	1.75	1.50
3	TTW+m	MT20	3.0	4.0		
4	TMVW-t	MT20	3.0	5.0	1.50	2.00
6	BMV1+p	MT20	2.0	4.0		
7	BMWWW-t	MT20	3.0	6.0		
8	BMWW-t	MT20	3.0	4.0		
9	BMV1+p	MT20	2.0	4.0		
	•					

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

### DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY **BUILDING DESIGNER**

BEA	BEARINGS											
	FACTO	DRED MAXIMUM FACTORED			INPUT	REQRD						
	GROSS R	EACTION	GROSS REACTION			BRG	BRG					
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX					
9	432	0	463	-140	-217	5-8	5-8					
6	606	0	627	0	-311	5-8	5-8					

PROVIDE ANCHORAGE AT BEARING JOINT 9 FOR 217 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 6 FOR 311 LBS FACTORED UPLIFT

PROVIDE FOR 140 LBS FACTORED HORIZONTAL REACTION AT JOINT 9

### UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS								
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
9	304	211/0	0/0	0/0	76 / -215	93 / 0	0/0	
6	422	315 / 0	0/0	0/0	54 / -291	107 / 0	0/0	
HOR 9	IZONTAL RE	ACTIONS 0/0	0/0	0/0	94 / -100	0/0	0 /0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 9, 6

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED

<u>LOADING</u> TOTAL LOAD CASES: (11)

CHO	ORDS					W E	BS		
MAX	. FACTORED	FACTOR	RED				MAX. FACTO	DRED	
MEMB.	FORCE	VERT. LO	AD LC	1 MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PL	F) (	CSI (LC)	<b>UNBRAC</b>	)	(LBS)	CSI (LC)	
FR-TO		FROM	TO		LENGTH	FR-TO			
1- 2	-444 / 215	-77.3	-77.3	0.14 (7)	6.25	8- 2	-3 / 72	0.02 (11)	
2- 3	-303 / 202	-77.3	-77.3	0.19 (8)	6.25	2-7	-88 / 59	0.03(3)	
3- 4	-394 / 211	-77.3	-77.3	0.18 (8)	6.25	7-3	-7 / 52	0.02 (11)	
4- 5	0 / 40	-77.3	-77.3	0.19(1)	10.00	1-8	-116 / 361	0.08 (1)	
9- 1	-442 / 229	0.0	0.0	0.04(1)	7.81	7- 4	-97 / 335	0.07(1)	
6- 4	-612 / 320	0.0	0.0	0.06(1)	7.81				
9-8	-121 / 135	-17.5	-17.5	0.05 (11)	6.25				
8- 7	-143 / 389	-17.5	-17.5	0.08 (1)	6.25				
7-6	-8 / 18	-17.5	-17.5	0.04 (11)	10.00				

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT { 40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

### **DESIGN CRITERIA**

SPECIFIED LOADS: LL = DL = LL = DL = AD = CH. 23.3 PSF 3.0 0.0 7.0 PSF PSF TOTAL LOAD 33.3 PSF

### SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

TOTAL WEIGHT = 38 lb

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09

- TPIC 2011

DESIGN ASSUMPTIONS -OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.31\*) CALCULATED VERT. DEFL.(LL)= L/999 (0.01\*) ALLOWABLE DEFL.(TL)= L/360 (0.31\*) CALCULATED VERT. DEFL.(TL)= L/999 (0.01\*)

CSI: TC=0.19 (2-3:8) , BC=0.08 (7-8:1) , WB=0.08 (1-8:1) , SSI=0.13 (2-3:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.64 (3) (INPUT = 0.90 ) JSI METAL= 0.16 (4) (INPUT = 1.00 )



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

JOB NAME TRUSS NAME QUANTITY DRWG NO. TR-GREENPARK-LECCO RIDGE-BLOCK 327 Page 96 of 159 TRUSS DESC TW0317-048 TW0317-048 T37 Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:50 2017 Page Kott Lumber Uxbridge, Stouffville, ON, TW ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-9zPpOOj23asHqm\_?psPrCaH6YxzyET1Up1fWUIzcJJh 1-3-8, 1-11-5 280 12-2-11 SCALE = 1:66.4 6 3x6 < 4x4 / 3x4 < 8 6.00 12 5x8 = 4x6 = 4x6 < 4x6 > WIO W5 9 2-1-10 Ş 5 R1 <sup>13</sup> 12 15 10 16 14 11 2x4 3x8 4x5 3x5 3x10 = 3x6 4x6 = 1.3-8 5-8 30-5-0 31-2-0 TOTAL WEIGHT = 121 DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY LUMBER N. L. G. A. CHORDS BUILDING DESIGNER **DESIGN CRITERIA** LUMBER DESCR SIZE BEARINGS FACTORED 3 DRY No.2 SPF SPF MAXIMUM FACTORED INPLIT REQRD SPECIFIED LOADS: No.2 GROSS REACTION GROSS REACTION CH. LL =
DL =
LL =
DL =
AD = HORZ UPLIFT IN-SX 3.0 2x4 DRY No 2 VFRT HOR7 DOWN IN-SX PSF 2x4 No.2 1618 221 5-8 5-8 HANGER BY OTHERS 9 2x4 DRY No.2 SPF 10 1478 1510 -635 7.0 **PSF** DRY DRY 2 No.2 MIN. SEAT SIZE: 3-8 TOTAL LOAD 33.3 PSF 10 -2x4 No.2 PROVIDE ANCHORAGE AT BEARING JOINT 17 FOR 730 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 10 FOR 635 LBS FACTORED UPLIFT 13 2v4 DRY No 2 SPACING = 24.0 IN. C/C 13 -10 No.2 ALL WEBS EXCEPT 2x3 DRY No.2 SPF ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12 SHALL BE PROVIDED BY BUILDG. DESIGNER DRY: SEASONED LUMBER. PROVIDE FOR 221 LBS FACTORED HORIZONTAL REACTION AT JOINT 17 THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010 UNFACTORED REACTIONS SNOW LIVE 0/0 COMBINED DEAD SOIL THIS DESIGN COMPLIES WITH: 789/0 88 / -727 - PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 PLATES (table is in inches) LEN Y 10 1037 726 / 0 0/0 0/0 79 / -654 312/0 0/0 - CSA 086-09 TMVW-t MT20 6.0 1.50 3.00 1.75 2.00 - TPIC 2011 TTWW-m MT20 HORIZONTAL REACTIONS (55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED TTWW-m MT20 4 0 6.0 2 25 2 75 0/0 0/0 0/0 158 / -132 0/00/04.0 2.00 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 17 5.0 TTW+p MT20 1.50 ROOF LIVE LOAD 3.0 3.0 4.0 TS-t TMWW-t MT20 6.0 ALLOWABLE DEFL.(LL)= L/360 (1.04")
CALCULATED VERT. DEFL.(LL) = L/999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (1.04")
CALCULATED VERT. DEFL.(TL) = L/999 (0.30") MT20 1.50 1.75 1.75 2.75 4.0 <u>BRACING</u> TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.87 FT. TMVW-t MT20 6.0 BMV1+p 2.0 4.0 10.0 1.50 3.75 MAX. UNBRACED BOTTOM CHORD LENGTH = 5.33 FT. OR RIGID CEILING DIRECTLY BMWW-t MT20 4.0 3.0 BMWWW-t MT20 6.0 1.75 3.00 MT20 ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED. CSI: TC=0.73 (8-9:8), BC=0.60 (14-15:1), BS-1 6.0 BMWW-t WB=0.76 (4-14:3), SSI=0.24 (8-9:1) MT20 3.0 5.0 1.50 2.25 BMWW-t 1.75 1.50 1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 5-12. DBS = 14-0-0 . CBF = 85 LBS. DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 16 BMWW-t MT20 3.0 8.0 1.50 3.50 1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 8-12. DBS = 20-0-0 . CBF = 80 LBS. BMV1+p MT20 COMP=1.10 SHEAR=1.10 TENS= 1.10 DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12. COMPANION LIVE LOAD FACTOR = 0.50 A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED. END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT **LOADING** TOTAL LOAD CASES: (11) NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN CHORDS WEBS MAX FACTORED FACTORED MAX FACTORED VERT. LOAD LC1 MAX MAX.
(PLF) CSI (LC) UNBRAC MEMB. FORCE MEMB. FORCE 618 354 1667 822 2284 1656 (LBS) (LBS) CSI (LC) FR-TO FROM -77.3 LENGTH FR-TO 10.00 16- 3 ΤΌ PLATE PLACEMENT TOL. = 0.250 inches 0/23 -77.3 0.10 (1) -585 / 261 0.09 (1) 2-3 3-4 4-5 -77.3 0.16 (7) -77.3 0.26 (7) -77.3 0.58 (7) -1734 / 796 -77.3 -77.3 5.07 3-15 -815 / 2021 0.45 (1) 0.17 (1) PLATE ROTATION TOL. = 5.0 Deg. -3159 / 1392 T.L. WISE TO 100083566 -77.3 -77.3 -77.3 JSI GRIP= 0.90 (15) (INPUT = 0.90) 4-14 -2496 / 1098 3.96 -1014 / 544 0.76(3)5- 6 6- 7 7- 8 -1671 / 797 -1682 / 816 -77.3 0.54 (7) -77.3 0.70 (8) 4.72 4.47 14- 5 5-12 -82 / 413 -1026 / 679 JSI METAL= 0.60 (13) (INPUT = 1.00) -77.3 -77.3 12- 6 12- 8 -1682 / 816 -77.3 0.70 (8) 4.47 -479 / 1083 0.62(7)8- 9 17- 2 -2240 / 938 -77.3 0.73 (8) -687 / 552 -1607 / 745 0.0 0.0 0.16(1) 6.57 11-8 -77 / 185 0.04 (8) 0.37 (1) 0.73 (8) 10- 9 -1457 / 670 0.0 0.0 0.14 (1) -678 / 1671 6.82 -732 / 2029 -17.5 -<u>17.</u>5 17-16 -204 / 187 -17.5 0.04 (8) 6 25 6.25 5.33 16-15

> **RECEIVED** TOWN OF MILTON

RESSU**REAOR (299**0) **25F1 7**TT

MAY BE LOCATED ON EAST-ULA PHAGE BALVING NON

EXTERNAL PEAK ING SYSTEM : M9ERNAL

100083566

NCE OF ON

March 10, 2017

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WIN

READ ALL NOTES ON THIS PAGE AND ON THE

IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED

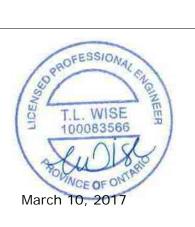
IN THE DESIGN OF THIS COMPONENT.

ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE

JOB PESC TR-GREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY DRWG NO Page 97 of 159 TRUSS DESC TW0317-048 TW0317-048 T38 Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:50 2017 Page 1 Kott Lumber Uxbridge, Stouffville, ON, TW ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-9zPpOOj23asHqm\_?psPrCaH6Yx?vEQQUp1fWUIzcJJh 1.3-8 4.7.5 280 9-6-11 SCALE = 1:66.4 6 3x4 \\ 3x6 < 3x4 < 5x8 = 8 4x6 = 6.00 12 3 4x6 > 4x6 < 9 3510 WIO 9 WЗ Ş 5 <sup>13</sup> 12 10 16 15 14 11 2x4 2x4 4x5 = 3x5 = 3x10 = 4x4 = 3x6 4x6 = 30-5-0 3-8 31-2-0 TOTAL WEIGHT = 126 lb LUMBER N. L. G. A. RULES CHORDS SIZE DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER **DESIGN CRITERIA** LUMBER DESCR BEARINGS FACTORED 3 DRY No.2 SPF SPF MAXIMUM FACTORED INPLIT REQRD SPECIFIED LOADS: LL = DL = LL = DL = AD = No.2 GROSS REACTION GROSS REACTION CH. PSF HORZ UPLIFT IN-SX 3.0 6 7 2x4 DRY No 2 SPF VFRT HOR7 DOWN IN-SX PSF 2x4 No.2 1618 221 -730 5-8 5-8 HANGER BY OTHERS 9 2x4 DRY No.2 SPF 10 1478 0 1510 0 -635 7.0 PSF DRY DRY SPF SPF 17 -2 No.2 MIN. SEAT SIZE: 3-8 TOTAL LOAD 33.3 PSF 10 -2x4 No.2 PROVIDE ANCHORAGE AT BEARING JOINT 17 FOR 730 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 10 FOR 635 LBS FACTORED UPLIFT 13 2v4 DRY No 2 SPE SPACING = 24.0 IN. C/C 13 -10 No.2 ALL WEBS EXCEPT 2x3 DRY No.2 SPF ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12 SHALL BE PROVIDED BY BUILDG. DESIGNER DRY: SEASONED LUMBER. PROVIDE FOR 221 LBS FACTORED HORIZONTAL REACTION AT JOINT 17 THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010 UNFACTORED REACTIONS SNOW LIVE 0/0 COMBINED DEAD SOIL THIS DESIGN COMPLIES WITH: 88 / -727 - PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09 789 / 0 0/0 PLATES (table is in inches) **PLATES** LEN Y 10 1037 726 / 0 0/0 0/0 79 / -654 312/0 0/0 TMVW-t MT20 6.0 1.75 2.75 2.00 3.25 - TPIC 2011 TTWW-m MT20 HORIZONTAL REACTIONS 6.0 4.0 (55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED TTWW-m MT20 4.0 2 25 3 00 0/0 0/0 0/0 158 / -132 0/00/03.0 MT20 1.75 0.75 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 17 TTW+p MT20 6.0 ROOF LIVE LOAD MT20 MT20 3.0 3.0 4.0 TS-t TMWW-t 6.0 ALLOWABLE DEFL.(LL)= L/360 (1.04\*) CALCULATED VERT. DEFL.(LL)= L/999 (0.14\*) ALLOWABLE DEFL.(TL)= L/360 (1.04\*) CALCULATED VERT. DEFL.(TL)= L/999 (0.25\*) 4.0 6.0 1.50 1.75 1.75 2.75 <u>BRACING</u> TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.89 FT. TMVW-t MT20 BMV1+p 2.0 4.0 10.0 1.50 3.75 MAX. UNBRACED BOTTOM CHORD LENGTH = 5.82 FT. OR RIGID CEILING DIRECTLY BMWW-t MT20 4.0 3.0 BMWWW-t MT20 6.0 1.75 3.00 MT20 ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED. CSI: TC=0.73 (8-9:8), BC=0.48 (14-15:1), BS-t 6.0 3.0 4.0 5.0 4.0 BMWW-t MT20 WB=0.93 (5-12:3), SSI=0.24 (8-9:1) BMWW-t MT20 1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 8-12. DBS = 20-0-0 . CBF = 80 LBS. DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 4.0 BMWW-t MT20 5.0 1.75 1.50 DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12. BMV1+p MT20 2.0 4.0 COMP=1.10 SHEAR=1.10 TENS= 1.10 A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH COMPANION LIVE LOAD FACTOR = 0.50

5 10 11 16

TEE-LOK TL20 PLATES IS ALLOWED.



END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

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LOADING TOTAL LOAD CASES: (11)

	ORDS					WE	BS		
MAX	. FACTORED	FACTOR	RED				MAX. FACTO	ORED	
MEMB.	FORCE	VERT. LO.	AD LC1	I MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PL	F) (	CSI (LC)	<b>UNBRAC</b>	)	(LBS)	CSI (LC)	
FR-TO		FROM	TO		LENGTH	FR-TO		, ,	
1- 2	0 / 23	-77.3	-77.3	0.10(1)	10.00	16-3	-265 / 178	0.05 (1)	
2-3	-2118 / 932	-77.3	-77.3	0.38 (7)	4.48	3-15	-473 / 1181	0.26 (1)	
3- 4	-2631 / 1203	-77.3	-77.3	0.23 (7)	4.22	15- 4	-871 / 416	0.17 (1)	
4-5	-2270 / 1027	-77.3	-77.3	0.40 (7)	4.33	4-14	-737 / 432	0.41 (3)	
5- 6	-1664 / 811	-77.3	-77.3	0.37 (7)	4.93	14- 5	-179 / 491	0.11 (1)	
6- 7	-1681 / 817	-77.3	-77.3	0.70 (8)	4.47	5-12	-904 / 602	0.93 (3)	
7-8	-1681 / 817	-77.3	-77.3	0.70 (8)	4.47	12-6	-520 / 1123	0.68 (7)	
8- 9	-2240 / 938	-77.3	-77.3	0.73 (8)	3.89	12-8	-688 / 550	0.31 (4)	
17- 2	-1581 / 754	0.0	0.0	0.16(1)	6.61	11-8	-76 / 186	0.04 (8)	
10-9	-1457 / 669	0.0	0.0	0.14(1)	6.82	2-16	-713 / 1914	0.42(1)	
						11-9	-732 / 2029	0.73 (8)	
17-16	-204 / 187	-17.5	-17.5	0.08 (11	) 6.25				
16-15	-900 / 1919	-17.5	-17.5	0.32(1)	6.25				
15-14	-1211 / 2691	<u>-17.</u> 5	-17.5	0.48(1)	5.82				
14-13	85 / 2005			(	6.25				
13-12	/2 /2		- 5 - 5	0.4 1)	6.25				
12-11	- / 2	17	- 5 - 5 - 5	0.4 1)					
11-10	_ /1	17	- 5	0.2 11	) 10.00		D	ECEN/E	$\overline{}$
_							K	ECEIVE	U
							_ TOW	'N OF MIL	T(
WINE	READ ALL NO	TES ON T	LIC D	GE AN	ON TH	IE	PRESSURE	OF { 9.0} PSF	
{40- <b>4</b> _	NGINEERING							<b>4LRP129</b> 5, 20°	
COE	INGINEERING	INUIEPA	GE EI	WF-1. II	JE NOT	FAGE	STING SYST	TEM}.INTERN	ΑL

IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED

IN THE DESIGN OF THIS COMPONENT.

ŎΝ ING SYSTEM).INTERNAL MAY BE LOOK #50700N EAST (0-0) FT-IN-SX AWAY
BUILDING DIVISION TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (9) (INPUT = 0.90) JSI METAL= 0.59 (9) (INPUT = 1.00 )

POBPES GREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY PLY DRWG NO. Page 98 of 159 T39 TW0317-048 TW0317-048 Kott Lumber Uxbridge, Stouffville, ON, TW Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:51 2017 Page ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-d9zBbkjgqu\_8SvZBNaw4koqJ6LLFzwid1hO40kzcJJg 1-3-8 5-11-5 7-3-5 17-11-7 SCALE = 1:52.5 2x4 | 5x8 = 3x4 = **3x6** = 5x8 = 5 7 8 6 6.00 12 4x5 🥟 4x6 < TI 🐼 2x4 || 13 15 14 12 11 16 10 **3x6** = 4x5 = 3x4 = 3x5 = 4x8 = **3x6** = 2x4 || 1-3-8 30-5-0 31-2-0 TOTAL WEIGHT = 120 lb DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER **DESIGN CRITERIA** SPECIFIED LOADS: CH. 23.3 3.0 PSF

LUMBER				
N. L. G. A. R	ULES			
CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 6	2x4	DRY	No.2	SPF
6 - 8	2x4	DRY	No.2	SPF
8 - 9	2x4	DRY	No.2	SPF
16 - 2	2x4	DRY	No.2	SPF
10 - 9	2x4	DRY	No.2	SPF
16 - 13	2x4	DRY	No.2	SPF
13 - 10	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PL/	PLATES (table is in inches)											
JT	TYPE	PLATES	W	LEN	Υ	Χ						
2	TMV+p	MT20	2.0	4.0								
3	TMWW-t	MT20	4.0	5.0	1.50	2.50						
4	TTWW-m	MT20	5.0	8.0	2.25	3.50						
5	TMWW-t	MT20	3.0	4.0								
6	TS-t	MT20	3.0	6.0								
7	TMW+w	MT20	2.0	4.0								
8	TTWW-m	MT20	5.0	8.0	2.25	3.25						
9	TMVW-t	MT20	4.0	6.0	1.75	Edge						
10	BMV1+p	MT20	2.0	4.0								
11	BMWW-t	MT20	3.0	6.0	1.50	1.75						
12	BMWWW-t	MT20	4.0	8.0								
13	BS-t	MT20	3.0	6.0								
14	BMWW-t	MT20	3.0	5.0	1.50	2.25						
15	BMWW-t	MT20	3.0	4.0								
16	BMVW1-t	MT20	4.0	5.0	1.50	1.75						

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

	FACTOR	RED	MAXIMU	M FACT	INPUT	REQRD	
	GROSS RE	ACTION	GROSS I	REACTIO	BRG	BRG	
JΤ	VERT	HORZ	DOWN	HORZ	<b>UPLIFT</b>	IN-SX	IN-SX
16	1583	0	1637	174	-819	5-8	5-8
10	1478	0	1513	0	-750	HANGER E	BY OTHERS
						MIN. SEAT	SIZE: 3-8

PROVIDE ANCHORAGE AT BEARING JOINT 16 FOR 819 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 10 FOR 750 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES. SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 174 LBS FACTORED HORIZONTAL REACTION AT JOINT 16

# UNFACTORED REACTIONS AND AND COMPONENT REACTIONS

	IST LUASE	IVIAA./I	VIIIA. COME ON	ENT KEACTION	JING		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
16	1109	789 / 0	0/0	0/0	136 / -791	320 / 0	0/0
10	1037	726 / 0	0/0	0/0	87 / -736	312 / 0	0/0
HOR 16	IZONTAL REA	ACTIONS 0/0	0/0	0/0	124 / -83	0/0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 16

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.81 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 5.71 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (11)

СН	ORDS					W E	BS		
MAX	(. FACTORED	FACTO	RED				MAX. FACTO	DRED	
MEMB.	FORCE	VERT. LC	AD LC	1 MAX	MAX.	MEMB	. FORCE	MAX	
	(LBS)	(PL	_F) (	CSI (LC)	<b>UNBRAC</b>	0	(LBS)	CSI (LC)	
FR-TO		FROM	TO		LENGTH	FR-TO			
1- 2	0 / 23	-77.3	-77.3	0.10(1)	10.00	3-15	0 / 148	0.03 (7)	
2-3	-9 / 114	-77.3	-77.3	0.15 (7)	10.00	15- 4	0 / 105	0.04 (11)	
3- 4	-2165 / 1140	-77.3	-77.3	0.29(7)	4.53	4-14	-558 / 921	0.60 (8)	
4- 5	-2666 / 1444	-77.3	-77.3	0.55 (1)	3.81	14- 5	-457 / 402	0.15(1)	
5-6	-2574 / 1380	-77.3	-77.3	0.54(1)	3.87	5-12	-117 / 98	0.12(3)	
6- 7	-2574 / 1380	-77.3	-77.3	0.54 (1)	3.87	12- 7	-527 / 407	0.17 (1)	
7-8	-2574 / 1379	-77.3	-77.3	0.54 (1)	3.88	12-8	-652 / 1164	0.70 (7)	
8- 9	-1847 / 931	-77.3	-77.3	0.61 (1)	4.30	11-8	-298 / 261	0.10 (1)	
	-226 / 204							0.74 (1)	
10-9	-1470 / 780	0.0	0.0	0.16 (1)	6.80	11- 9	-723 / 1706	0.46 (8)	
	-1003 / 1873			0.40 (1)					
	-883 / 1935			0.41 (1)					
	-1259 / 2672			0.47 (1)					
	-1259 / 2672			0.47 (1)					
	-665 / 1647								
11-10	-15 / 32	-17.5	-17.5	0.15 (11	) 6.25				

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

**RECEIVED** TOWN OF MILTON MAR 29, 2017 17-4978 **BUILDING DIVISION** 

LL = DL = LL = DL = AD = 7.0 PSF TOTAL LOAD 33.3

### SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09 - TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.04") CALCULATED VERT. DEFL.(LL)= L/ 999 (0.15") ALLOWABLE DEFL.(TL)= L/360 (1.04") CALCULATED VERT. DEFL.(TL)= L/999 (0.27")

CSI: TC=0.61 (8-9:1), BC=0.47 (12-14:1), WB=0.74 (3-16:1), SSI=0.22 (4-5:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

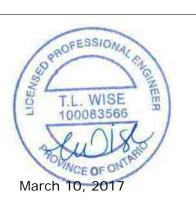
### NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (14) (INPUT = 0.90) JSI METAL= 0.76 (13) (INPUT = 1.00)



JOB PESC TR-GREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY DRWG NO Page 99 of 159 TRUSS DESC TW0317-048 TW0317-048 T40 Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:51 2017 Page Kott Lumber Uxbridge, Stouffville, ON, TW  $ID: 4BORDhR74Ei? Dog\_xdfUkJyKZi\_-d9zBbkjgqu\_8SvZBNaw4koqKWLNnz?Md1hO40kzcJJ \\ \label{eq:bordhR74} \\ d1hO40kzcJJ \\ \label{eq:bordhR74} \\ d2hO40kzcJJ \\ \label{eq:bordhR74} \\ d3hO40kzcJJ \\ \label{eq:bordhR74} \\ d4hO40kzcJJ \\ \label{eq:bordhR74} \\$ <u>1-3-</u>8 8-7-5 9-11-5 12-7-7 SCALE = 1:53.7 4x6 = 2x4 || AYR > 4 5 6 6.00 12 3x4 < 3x4 / 3 W6 6-1-10 W5 W5 4x6 < 8 4x6 / WЗ 10 8 **B**1 **B2** 17 14 12 16 15 13 11 10 9 3x6 = 3x6 = 3x4 = 2x4 || 4x5 = 3x4 = 3x6 = 2x4 || 3x8 1-3-8 30-5-0 5-8 31-2-0 TOTAL WEIGHT = 126 lb LUMBER N. L. G. A. RULES CHORDS SIZE DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER **DESIGN CRITERIA** LUMBER DESCR BEARINGS FACTORED 4 DRY No.2 SPF SPF MAXIMUM FACTORED INPLIT REQRD SPECIFIED LOADS: LL = DL = LL = DL = AD = No.2 GROSS REACTION GROSS REACTION CH. PSF BRG HORZ UPLIFT IN-SX 6 -17-3.0 2x4 DRY No 2 SPF VFRT HOR7 DOWN IN-SX PSF 8 2 8 2x4 No.2 1633 200 5-8 5-8 HANGER BY OTHERS 2x4 DRY No.2 SPF 9 1478 0 1503 -725 7.0 PSF 17 -14 -14 12 No.2 MIN. SEAT SIZE: 3-8 TOTAL LOAD 33.3 PSF 2x4 No.2 PROVIDE ANCHORAGE AT BEARING JOINT 17 FOR 797 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 9 FOR 725 LBS FACTORED UPLIFT 12 -9 DRY No.2 SPF SPACING = 24.0 IN. C/C ALL WEBS DRY SPF 2x3 No.2 ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, **EXCEPT** LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12 SHALL BE PROVIDED BY BUILDG. DESIGNER DRY: SEASONED LUMBER. PROVIDE FOR 200 LBS FACTORED HORIZONTAL REACTION AT JOINT 17 THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010 UNFACTORED REACTIONS SNOW LIVE 0/0 PLATES (table is in inches)
JT TYPE PLATES COMBINED DEAD SOIL THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09 789 / 0 0/0 W LEN Y Y X 1.75 2.75 TMVW-1 MT20 4.0 6.0 1037 726 / 0 0/0 0/0 62 / -718 312/0 0/0 TMWW-1 MT20 4.0 1.50 1.75 - TPIC 2011 6.0 1.75 2.00 ΓTWW-m HORIZONTAL REACTIONS MT20 (55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED TMW+w MT20 2.0 4.0 3.0 4.0 2.0 3.0 4 0 0/0 0/0 0/0 143 / -101 0/00/0TTWW-m 8.0 4.0 MT20 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 17 1.50 1.75 TMWW-t MT20 ROOF LIVE LOAD TMVW-t BMV1+p MT20 MT20 6.0 1.75 Edge ALLOWABLE DEFL.(LL)= L/360 (1.04") CALCULATED VERT. DEFL.(LL)= L/999 (0.11") ALLOWABLE DEFL.(TL)= L/360 (1.04") CALCULATED VERT. DEFL.(TL)= L/999 (0.19") 4.0 <u>BRACING</u> TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.27 FT. BMWW-t MT20 6.0 1.50 1.75

10 BMWW-t 3.0 11 12 BS-t MT20 6.0 3.0 3.0 BMWWW-MT20 8.0 MT20 BS-t 6.0 BMWW-t MT20 3.0 4.0 BMWW-t 1.75 1.50 17 BMV1+p MT20 2.0 4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

T.L. WISE TO 100083566

100083566

WCE OF ON

March 10, 2017

MAX. UNBRACED BOTTOM CHORD LENGTH = 6.21 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 4-13, 6-13. DBS = 20-0-0. CBF = 45 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING TOTAL LOAD CASES: (11)

	20/12 0/1020.	()							
	O R D S	FACTOR	RED			WE		(, FACT(	ORED
MEMB.	FORCE	VERT. LO.	AD LC1	I MAX	MAX.	MEMB.		FORCE	MAX
	(LBS)	(PL	F) (	CSI (LC)	UNBRAC	0		(LBS)	CSI (LC)
FR-TO	( - /	FROM		- ( -,	LENGTH			/	( -/
1- 2	0 / 23	-77.3	-77.3	0.10(1)	10.00	16- 3	-249	/ 221	0.05 (1)
2-3	-2204 / 1094				4.40			3 / 278	0.16 (3)
3- 4	-2016 / 1070	-77.3	-77.3	0.43 (7)	4.58	15- 4	-94	/ 252	0.05 (1)
4- 5	-2110 / 1153				4.27			6 / 437	0.15 (8)
	-2110 / 1153	-77.3		0.52 (7)		13- 5		/ 488	0.36 (3)
6- 7	-1839 / 990	-77.3		0.35 (8)		13-6	-412	2 / 657	0.20 (7)
7-8	-1749 / 886	-77.3		0.33 (8)		11-6	-26	6 / 96	0.03 (11)
17-2	-1592 / 820	0.0	0.0	0.16(1)	6.60	11-7	-29	/ 138	0.03 (5)
9-8	-1467 / 746	0.0	0.0	0.16(1)	6.81	10-7	-475	7 306	0.12 (1)
						2-16	-871	/ 2007	0.44 (1)
17-16	-182 / 133	-17.5	-17.5	0.09 (11	) 6.25	10-8	-745	/ 1673	0.37 (1)
16-15	-1009 / 2012	-17.5	-17.5	0.37 (1)	6.21				
15-14	-782 / 1800	-17.5	-17.5	0.34(1)	6.25				
14-13	78" / 18			(	6.25				
13-12	/1	17	- 5	0.3 1) 0.3 1)	6.25				
12-11	- / 1		- 5 - 5		6.25				
11-10	<b>■</b> 67 / 1: 2	17	- 5	0.3 1)	6.25				COEN/E
10-9	15/32		5	0.0, 11	) 6.25				ECEIVE
_							_1	TOW	/N OF MILT
	DEAD ALL NO	71							

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED WIN IN THE DESIGN OF THIS COMPONENT.

TON

CSI: TC=0.52 (4-5:7), BC=0.37 (15-16:1),

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

WB=0.44 (2-16:1) , SSI=0.24 (4-5:1)

COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg.

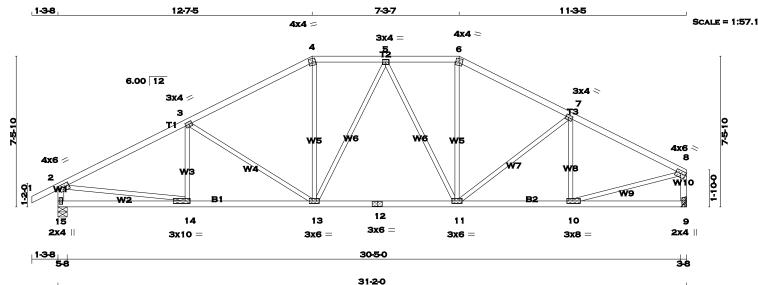
TRUSS MANUFACTURING PLANT

JSI GRIP= 0.90 (10) (INPUT = 0.90) JSI METAL= 0.59 (16) (INPUT = 1.00)

NAIL VALUES

RESSU**RÆAOR (299**0) **2.6F1**77T EXTERNAL PEAK ING SYSTEM): M9778NAL MAY BE LOCATED ON EABITHLED HINGS BAY WASHON

POBPES GREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY PLY DRWG NO. Page 100 of 159 TW0317-048 TW0317-048 T41 Kott Lumber Uxbridge, Stouffville, ON, TW Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:52 2017 Page ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-6MXap4klbC6?438NxHRJH?MUyliWiPPnGL8dYAzcJJf 11-3-5 12-7-5 7-3-7 1-3-8



LUMBER				
N. L. G. A. R	ULES			
CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 6	2x4	DRY	No.2	SPF
6 - 8	2x4	DRY	No.2	SPF
15 - 2	2x4	DRY	No.2	SPF
9 - 8	2x4	DRY	No.2	SPF
15 - 12	2x4	DRY	No.2	SPF
12 - 9	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				_

DRY: SEASONED LUMBER.

PL/	PLATES (table is in inches)									
JT	TYPE	PLATES	W	LEN	Υ	Χ				
2	TMVW-t	MT20	4.0	6.0	1.75	2.75				
3	TMWW-t	MT20	3.0	4.0	1.50	1.75				
4	TTW-m	MT20	4.0	4.0						
5	TMWW-t	MT20	3.0	4.0						
6	TTW-m	MT20	4.0	4.0						
7	TMWW-t	MT20	3.0	4.0	1.50	1.75				
8	TMVW-t	MT20	4.0	6.0	1.75	Edge				
9	BMV1+p	MT20	2.0	4.0						
10	BMWW-t	MT20	3.0	8.0	1.50	3.50				
11	BMWWW-t	MT20	3.0	6.0						
12	BS-t	MT20	3.0	6.0						
13	BMWWW-t	MT20	3.0	6.0						
14	BMWW-t	MT20	3.0	10.0	1.50	3.00				
15	BMV1+p	MT20	2.0	4.0						

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS	AND LOADINGS SPEC	IFIED BY FABRICATOR	R TO BE VERIFIED BY
BUILDING DESIGNER			
DEADINGS			

DEA	EARINGS										
	FACTO	RED	MAXIMU	MAXIMUM FACTORED			REQRD				
	GROSS R	EACTION	GROSS	REACTIO	N	BRG	BRG				
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX				
15	1583	0	1626	226	-768	5-8	5-8				
9	1478	0	1500	0	-693		BY OTHERS T SIZE: 3-8				

PROVIDE ANCHORAGE AT BEARING JOINT 15 FOR 768 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 9 FOR 693 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES. SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 226 LBS FACTORED HORIZONTAL REACTION AT JOINT 15

UNF	UNFACTORED REACTIONS										
	1ST LCASE MAX./MIN. COMPONENT REACTIONS										
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND						
15	1109	789 / 0	0/0	0/0	108 / -75						
0	4007	700 / 0	0.10	0.10	FC / CO						

15	1109	789 / 0	0/0	0/0	108 / -754	320 / 0	0/0
9	1037	726 / 0	0/0	0/0	56 / -695	312 / 0	0/0
HOR	IZONTAL RE	ACTIONS					
15		0/0	0/0	0/0	161 / -120	0/0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 15

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.15 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 6.23 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

# LOADING TOTAL LOAD CASES: (11)

СН	CHORDS WEBS									
MA)	K. FACTORED	FACTO	RED				MAX. FACTO	ORED		
MEMB.	FORCE	VERT. LO	AD LC	1 MAX	MAX.	MEMB.	FORCE	MAX		
	(LBS)	(PL	.F) (	CSI (LC)	<b>UNBRAC</b>	2	(LBS)	CSI (LC)		
FR-TO		FROM	TO		LENGTH	FR-TO				
1-2	0 / 23	-77.3	-77.3	0.10(1)	10.00	14- 3	-152 / 201	0.04(1)		
2-3	-2227 / 1059	-77.3	-77.3	0.60 (7)	4.15	3-13	-512 / 442	0.57(3)		
3- 4	-1816 / 939	-77.3	-77.3	0.59 (7)	4.55	13- 4	-214 / 492	0.21 (8)		
4- 5	-1612 / 923	-77.3	-77.3	0.24 (7)	5.14	13- 5	-131 / 214	0.17 (4)		
5-6	-1530 / 880	-77.3	-77.3	0.24 (8)	5.24	5-11	-290 / 275	0.39(3)		
6- 7	-1729 / 900	-77.3	-77.3	0.49 (8)	4.75	11-6	-220 / 487	0.22 (7)		
7-8	-1850 / 894	-77.3	-77.3	0.49 (8)	4.60	11- 7	-238 / 292	0.23 (4)		
15-2	-1578 / 798	0.0	0.0	0.16(1)	6.62	10- 7	-342 / 265	0.11(1)		
9-8	-1457 / 721	0.0	0.0	0.16(1)	6.82	2-14	-817 / 2022	0.58 (7)		
						10-8	-717 / 1735	0.41 (8)		
15-14	-208 / 159	-17.5	-17.5	0.15 (11	) 6.25					
14-13	-995 / 2045	-17.5	-17.5	0.40(1)	6.23					
13-12	-641 / 1668	-17.5	-17.5	0.35 (1)	6.25					
12-11	-641 / 1668	-17.5	-17.5	0.35(1)	6.25					
11-10	-663 / 1678	-17.5	-17.5	0.35(1)	6.25					
10-9	-15 / 32	-17.5	-17.5	0.12 (11	) 6.25					

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT (40-0-0) FIN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK
COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL
WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

**RECEIVED** TOWN OF MILTON MAR 29, 2017 17-4978 **BUILDING DIVISION** 

SOIL

DEAD

### **DESIGN CRITERIA**

SPECIFIED LOADS: TOP CH. LL = LL = 23.3 DL = 3.0 LL = 0.0 DL = 7.0 AD = 33.3 PSF TOTAL LOAD

### SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

TOTAL WEIGHT = 130 lb

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09

- TPIC 2011 (55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED

ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.04")
CALCULATED VERT. DEFL.(LL)= L/999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.04")
CALCULATED VERT. DEFL.(TL)= L/999 (0.19")

CSI: TC=0.60 (2-3:7) , BC=0.40 (13-14:1) , WB=0.58 (2-14:7) , SSI=0.21 (2-3:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (14) (INPUT = 0.90) JSI METAL= 0.60 (2) (INPUT = 1.00)



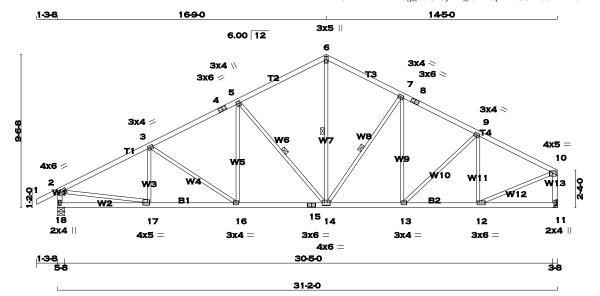
JOB PESC TR-GREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY TRUSS DESC TW0317-048 T42 Kott Lumber Uxbridge, Stouffville, ON, TW

DRWG NO. Page 101 of 159 TW0317-048

SCALE = 1:71.8

TOTAL WEIGHT = 136 lb

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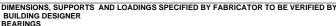


LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR 4 2x4 DRY No.2 No.2 SPF SPF 2x4 6 -8 -18-8 2x4 DRY No 2 SPF SPF 10 2x4 No.2 2 2x4 DRY No.2 DRY DRY SPF SPF 10 2x4 No.2 18 -15 2x4 No.2 15 -11 2x4 DRY No.2 SPF ALL WEBS DRY SPF 2x3 No.2 **EXCEPT** 

DRY: SEASONED LUMBER

PL/	PLATES (table is in inches)											
JT	TYPE	PLATES	W	LEN	Υ	X						
2	TMVW-t	MT20	4.0	6.0	1.75	2.75						
3, 7, 9												
3	TMWW-t	MT20	3.0	4.0	1.50	1.75						
4	TS-t	MT20	3.0	6.0								
5	TMWW+t	MT20	3.0	4.0	1.75	0.75						
6	TTW+p	MT20	3.0	5.0								
8	TS-t	MT20	3.0	6.0								
10	TMVW-p	MT20	4.0	5.0	1.50	2.50						
11	BMV1+p	MT20	2.0	4.0								
12	BMWW-t	MT20	3.0	6.0	1.50	2.00						
13	BMWW-t	MT20	3.0	4.0								
14	BMWWW-t	MT20	4.0	6.0								
15	BS-t	MT20	3.0	6.0								
16	BMWW-t	MT20	3.0	4.0								
17	BMWW-t	MT20	4.0	5.0	1.75	1.50						
12	RM\/1±n	MT20	2.0	4.0								

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.



BEAKINGS											
	FACTORED		MAXIMU	M FACT	ORED	INPUT	REQRD				
	GROSS R	GROSS	REACTIO	BRG	BRG						
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX				
18	1583	0	1616	288	-719	5-8	5-8				
11	1478	0	1507	0	-628	HANGER I	BY OTHERS				
						MIN. SEAT	SIZE: 3-8				

PROVIDE ANCHORAGE AT BEARING JOINT 18 FOR 719 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 11 FOR 628 LBS FACTORED UPLIFT

ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 288 LBS FACTORED HORIZONTAL REACTION AT JOINT 18

UNF	ACTORED REA	<u>ACTIONS</u>
	1ST LCASE	MAX./MIN

OINE	ONFACTORED REACTIONS										
	1ST LCASE	MAX./	MIN. COMPO	NENT REACTION	ONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL				
18	1109	789 / 0	0/0	0/0	83 / -719	320 / 0	0/0				
11	1037	726 / 0	0/0	0/0	73 / -649	312 / 0	0/0				
HOR	HORIZONTAL REACTIONS										
18		0/0	0/0	0/0	206 / -148	0/0	0 /0				

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 18

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.32 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 5-14, 6-14, 7-14. DBS = 20-0-0 . CBF = 84 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING TOTAL LOAD CASES: (11)

7.5.7.12.267.15 67.62.5. (1.1)								
СН	ORDS					WE	BS	
MAX	. FACTORED	FACTOR	RED				MAX. FACTO	ORED
MEMB.	FORCE	VERT. LO.	AD LC	1 MAX	MAX.	MEMB.	FORCE	MAX
	(LBS)	(PL	F) (	CSI (LC)	<b>UNBRAC</b>		(LBS)	CSI (LC)
FR-TO	. ,	FROM					, ,	, ,
1- 2	0 / 23	-77.3	-77.3	0.10(1)	10.00	17-3	-189 / 192	0.05 (1)
2-3	-2188 / 960	-77.3	-77.3	0.43 (7)	4.32	3-16	-326 / 309	0.26 (3)
3- 4	-1918 / 907	-77.3	-77.3	0.42 (7)	4.63	16- 5	-106 / 277	0.08 (7)
4- 5	-1918 / 907	-77.3	-77.3	0.42 (7)	4.63	5-14	-713 / 539	0.34 (3)
5- 6	-1437 / 755					14- 6	-475 / 940	0.28 (7)
6- 7	-1436 / 784	-77.3	-77.3	0.30 (8)	5.27	14- 7	-421 / 404	0.19 (4)
7-8	-1669 / 796	-77.3	-77.3	0.33 (8)	4.97	13- 7	-37 / 80	0.03 (8)
8- 9	-1669 / 796	-77.3	-77.3	0.33 (8)	4.97	13- 9	-25 / 135	0.03 (8)
9-10	-1601 / 703	-77.3	-77.3	0.32 (8)	5.04	12-9	-490 / 284	0.17 (1)
18- 2	-1572 / 746	0.0	0.0	0.16(1)	6.63	2-17	-734 / 1984	0.44 (1)
11-10	-1471 / 652	0.0	0.0	0.18 (1)	6.79	12-10	-562 / 1543	0.34 (1)
18-17	-270 / 198	-17.5	-17.5	0.12 (11	) 6.25			
17-16	-967 / 2014	-17.5	-17.5	0.36(1)	6.25			
16-15	-711 / 1744			0.33(1)				
15-14	-711 / 1744			0.33 (1)				
14-13	-451 / 1490			0.29 (1)			В	ECEN/ED
13-12	-480 / 1429	-17.5	-17.5	0.27 (1)	6.25			ECEIVED
12-11	-20 / 42	-17.5	-17.5	0.10 (11	) 6.25		TOW	N OF MILTON
							D.4	AD 20 2047
							IVI	AR 29, 2017

**DESIGN CRITERIA** 

SPECIFIED LOADS: LL = DL = LL = DL = AD = CH. PSF 3.0 PSF PSF 7.0 PSF TOTAL LOAD 33.3 PSF

### SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012, BCBC 2012, ABC 2014

- CSA 086-09 - TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.04") CALCULATED VERT. DEFL.(LL) = L/999 (0.10") ALLOWABLE DEFL.(TL)= L/360 (1.04") CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.43 (2-3:7) , BC=0.36 (16-17:1) , WB=0.44 (2-17:1) , SSI=0.18 (2-3:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

17-4978 **BUILDING DIVISION** 

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (10) (INPUT = 0.90 ) JSI METAL= 0.59 (17) (INPUT = 1.00 )



OB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. TR-GREENPARK-LECCO RIDGE-BLOC TRUSS DESC.	K 327	DRWG NO. Pag	e 102 of 159
ΓW0317-048	T42	1	1			TWO	)317-048
ott Lumber Uxbridge, Stouffville	e, ON, TW			Version 8.100 S ID:4BORDhR74Ei?Dog_xdfUkJyKZ	Feb 9 2017 M	iTek Industries, Inc. Fri Ma	r 10 14:20:52 2017 Page 2
				ID:4BORDhR74Ei?Dog_xdfUkJyK2	Zi6MXap4l	dbC6?438NxHRJH?M	XkljCiSbnGL8dYAzcJJ
		WIND LOAD AF	PLIED IS DE	ERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF NCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK SED ON THE {MAIN WIND FORCE RESISTING SYSTEM},INTERN, ED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON USS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AV	AT		
		(40-0-0) F1-IN-	SX REFERE S CoCa BA	SED ON THE (MAIN WIND FORCE RESISTING SYSTEM) INTERNA	ΔΙ		
		WIND PRESSU	JRE IS BASE	ED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON			
		(OPEN TERRA	.IN}, AND TR	USS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AV	VAY		
		FROM EAVE.					



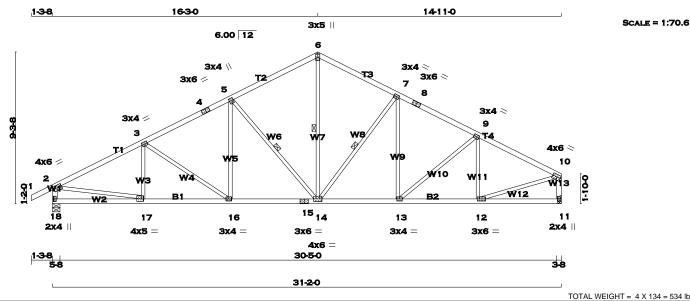
READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

JOB PESC TR-GREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY DRWG NO. TRUSS DESC TW0317-048 T43 Kott Lumber Uxbridge, Stouffville, ON, TW

Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:53 2017 Page 1 ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-aY5y0QlwMVEshDjaU?yYqDvim93WRurwV?tB5dzcJJe

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TW0317-048



LUMBER				
N. L. G. A. R	ULES			
CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 6	2x4	DRY	No.2	SPF
6 - 8	2x4	DRY	No.2	SPF
8 - 10	2x4	DRY	No.2	SPF
18 - 2	2x4	DRY	No.2	SPF
11 - 10	2x4	DRY	No.2	SPF
18 - 15	2x4	DRY	No.2	SPF
15 - 11	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)										
JT	TYPE	PLATES	W	LEN	Υ	Χ				
2	TMVW-t	MT20	4.0	6.0	1.75	2.75				
3, 7	', 9									
3	TMWW-t	MT20	3.0	4.0	1.50	1.75				
4	TS-t	MT20	3.0	6.0						
5	TMWW+t	MT20	3.0	4.0	1.75	0.75				
6	TTW+p	MT20	3.0	5.0						
8	TS-t	MT20	3.0	6.0						
10	TMVW-t	MT20	4.0	6.0	1.75	Edge				
11	BMV1+p	MT20	2.0	4.0						
12	BMWW-t	MT20	3.0	6.0	1.50	1.75				
13	BMWW-t	MT20	3.0	4.0						
14	BMWWW-t	MT20	4.0	6.0						
15	BS-t	MT20	3.0	6.0						
16	BMWW-t	MT20	3.0	4.0						
17	BMWW-t	MT20	4.0	5.0	1.75	1.50				
18	BMV1+p	MT20	2.0	4.0						

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED

# DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS

	FACTO	RED	MAXIMU	M FACT	ORED	INPUT	REQRD	
	GROSS R	EACTION	GROSS	REACTIO	N	BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
18	1583	0	1615	261	-715	5-8	5-8	
11	1478	0	1510	0	-636	HANGER BY OTHERS		
						MIN. SEAT SIZE: 3-8		

PROVIDE ANCHORAGE AT BEARING JOINT 18 FOR 715 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 11 FOR 636 LBS FACTORED UPLIFT

ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 261 LBS FACTORED HORIZONTAL REACTION AT JOINT 18

### UNFACTORED REACTIONS

	1ST LCASE	MAX.	/MIN. COMPON	<u>IENT REACTIO</u>	ONS		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
18	1109	789 / 0	0/0	0/0	80 / -716	320 / 0	0/0
11	1037	726 / 0	0/0	0/0	79 / -655	312 / 0	0/0
HOR	IZONTAL RE	ACTIONS 0/0	0/0	0/0	186 / -145	0/0	0 /0
10		0/0	070	0/0	100 / -145	070	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 18

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.34 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 5-14, 6-14, 7-14. DBS = 20-0-0 . CBF = 80 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING TOTAL LOAD CASES: (11)

		` '								
СН	ORDS					WE	вѕ			
MAX	K. FACTORED	FACTO	RED				MAX	. FACT	ORED	
MEMB.	FORCE	VERT. LO	AD LC	1 MAX	MAX.	MEMB.		FORCE	MAX	
	(LBS)	(PL	.F) (	CSI (LC)	<b>UNBRAC</b>			(LBS)	CSI (LC)	
FR-TO		FROM	TO		LENGTH	FR-TO				
1-2	0 / 23	-77.3	-77.3	0.10(1)	10.00	17-3	-201	/ 192	0.05 (1)	
2-3	-2182 / 952	-77.3	-77.3	0.41 (7)	4.34	3-16	-298	/ 293	0.22 (3)	
3-4	-1938 / 908	-77.3	-77.3	0.40 (7)	4.63	16- 5	-100	/ 259	0.07 (7)	
4- 5	-1938 / 908	-77.3	-77.3	0.40 (7)	4.63	5-14	-682	/ 520	0.30(3)	
5-6	-1480 / 771	-77.3	-77.3	0.37 (7)	5.13	14-6	-483	/ 980	0.26 (7)	
6- 7	-1480 / 789	-77.3	-77.3	0.32 (8)	5.19	14- 7	-510	/ 439	0.22 (4)	
7-8	-1788 / 839	-77.3	-77.3	0.35 (8)	4.82	13- 7	-58	/ 121	0.05 (8)	
8- 9	-1788 / 839			0.35 (8)		13- 9	-65	/ 182	0.05 (4)	
9-10	-1814 / 787	-77.3	-77.3	0.34 (8)	4.76	12-9	-387	/ 253	0.11(1)	
18- 2	-1573 / 741	0.0	0.0	0.16(1)	6.63	2-17	-730	/ 1980	0.44(1)	
11-10	-1471 / 661	0.0	0.0	0.16(1)	6.79	12-10	-629	/ 1690	0.38 (1)	
18-17	-243 / 194	-17.5	-17.5	0.12 (11						
17-16	-954 / 2008	-17.5	-17.5	0.36(1)	6.25					
16-15	-712 / 1762	-17.5	-17.5	0.33 (1)						
15-14	-712 / 1762	-17.5	-17.5	0.33 (1)						
14-13	-489 / 1594			0.31 (1)					FOEN/F	
13-12	-573 / 1616	-17.5	-17.5	0.30(1)					ECEIVE	
12-11	-15 / 32	-17.5	-17.5	0.10 (11	) 6.25		-	TOV	/N OF MIL	_T(
							-		4 D 00 00	
								M	AR 29, 20	17 /

**DESIGN CRITERIA** 

SPECIFIED LOADS: LL = 23.3 DL = 3.0 LL = 0.0 DL = 7.0 AD = 33.3 CH. PSF PSF TOTAL LOAD PSF

### SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012, BCBC 2012, ABC 2014 - CSA 086-09

- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.04")
CALCULATED VERT. DEFL.(LL) = L/999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.04") CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.41 (2-3:7) , BC=0.36 (16-17:1) , WB=0.44 (2-17:1) , SSI=0.18 (2-3:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

ON

17-4978 **BUILDING DIVISION** 

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (12) (INPUT = 0.90 ) JSI METAL= 0.59 (17) (INPUT = 1.00 )



TRUSS NAME
THUSS NAME
THUSS NAME
THUSS DESC.

TREGREENPARK-LECCO RIDGE-BLOCK 327
TRUSS DESC.

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Kott Lumber Uxbridge, Stouffville, ON, TW

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ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-aY5y0QlwMVEshDjaU?yYqDvim93WRurwV?tB5dzcJJe

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.0) PSF AT
(40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK
COEFFICIENTS, CpCg\_, BASED ON THE (MAIN WIND PORCE RESISTING SYSTEM).INTERNAL
WIND PRESSURE IS BASED ON DERIVED FROM RESISTING SYSTEM,INTERNAL
WIND PRESSURE IS BASED ON THE (MAIN WIND PORCE RESISTING SYSTEM).INTERNAL
WIND PRESSURE IS BASED ON DERIVED FROM RESISTING SYSTEM.INTERNAL
REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL
REFERENCE HEIGHT ABOVE GRADE AND USING EXTERN



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

JOB NAME TRUSS NAME

T44

QUANTITY PLY 6

JOB PR-GREENPARK-LECCO RIDGE-BLOCK 327 TRUSS DESC.

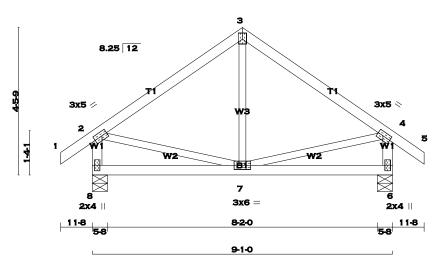
DRWG NO

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SCALE = 1:34.9

TW0317-048 Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:53 2017 Page 1 Kott Lumber Uxbridge, Stouffville, ON, TW ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-aY5y0QlwMVEshDjaU?yYqDvkM97UR\_pwV?tB5dzcJJe

> 11-8 4-6-8 11-8 3x4 II



LUMBER						
N. L. G. A. R	ULES					
CHORDS	SIZE		LUMBER	DESCR.		
1 - 3	2x4	DRY	No.2	SPF		
3 - 5	2x4	DRY	No.2	SPF		
8 - 2	2x4	DRY	No.2	SPF		
6 - 4	2x4	DRY	No.2	SPF		
8 - 6	2x4	DRY	No.2	SPF		
ALL WEBS	2x3	DRY	No.2	SPF		
EXCEPT						

DRY: SEASONED LUMBER.

PL/	ATES	(table is in inches)		
.IT	TYPF	PLATES	W	

JT	TYPE	PLATES	W	LEN	Υ	X
2	TMVW-t	MT20	3.0	5.0	1.50	2.00
3	TTW+p	MT20	3.0	4.0	2.25	1.50
4	TMVW-t	MT20	3.0	5.0	1.50	2.00
6	BMV1+p	MT20	2.0	4.0		
7	BMWWW-t	MT20	3.0	6.0		
8	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

### DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

DEAL	KINGS						
	FACTOR	RED	MAXIMUM FACTORED			INPUT	REQRD
	<b>GROSS RE</b>	ACTION	GROSS F	REACTIO	N	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
8	511	0	538	-209	-221	5-8	5-8
6	511	0	538	0	-221	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 8 FOR 221 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 6 FOR 221 LBS FACTORED UPLIFT

PROVIDE FOR 209 LBS FACTORED HORIZONTAL REACTION AT JOINT 8

### UNFACTORED REACTIONS

	1ST LCASE	MAX./I	MIN. COMPON	NENT REACTION	DNS		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
8	357	260 / 0	0/0	0/0	67 / -220	97 / 0	0/0
6	357	260 / 0	0/0	0/0	67 / -220	97 / 0	0/0
HOR 8	IZONTAL RE	ACTIONS 0/0	0/0	0/0	149 / -149	0/0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 8, 6

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (11)

CHO	DRDS					W E	BS	
MAX.	FACTORED	FACTO	RED				MAX. FACTO	RED
MEMB.	FORCE	VERT. LO	AD LC1	I MAX	MAX.	MEMB.	FORCE	MAX
	(LBS)	(PL	.F) (	CSI (LC)	<b>UNBRAC</b>	)	(LBS)	CSI (LC)
FR-TO		FROM	TO		LENGTH	FR-TO		, ,
1- 2	0 / 23	-77.3	-77.3	0.06(1)	10.00	7-3	-15 / 74	0.03 (11)
2-3	-327 / 167	-77.3	-77.3	0.31 (7)	6.25	2-7	-60 / 283	0.06 (1)
3- 4	-327 / 167	-77.3	-77.3	0.31 (8)	6.25	7- 4	-60 / 283	0.06 (1)
4- 5	0 / 23	-77.3	-77.3	0.06(1)	10.00			
8- 2	-507 / 242	0.0	0.0	0.05(1)	7.81			
6- 4	-507 / 242	0.0	0.0	0.05 (1)	7.81			
				. ,				
8- 7	-189 / 200	-17.5	-17.5	0.10 (11	6.25			
7-6	-9 / 20	-17.5	-17.5	0.10 (11	10.00			
					,			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CPCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

### **DESIGN CRITERIA**

SPEC	IFIED	LUADS	ri.	
TOP	CH.	LL =	23.3	PS
		DL =	3.0	PS
BOT	CH.	LL =	0.0	PS
			7.0	PS

TOTAL LOAD = 33.3 PSF

### SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

TOTAL WEIGHT = 6 X 38 = 229 lb

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014 - CSA 086-09 TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.30")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.30") CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.31 (2-3:7), BC=0.10 (6-7:11), WB=0.06 (2-7:1) , SSI=0.12 (2-3:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.47 (7) (INPUT = 0.90 ) JSI METAL= 0.14 (2) (INPUT = 1.00 )

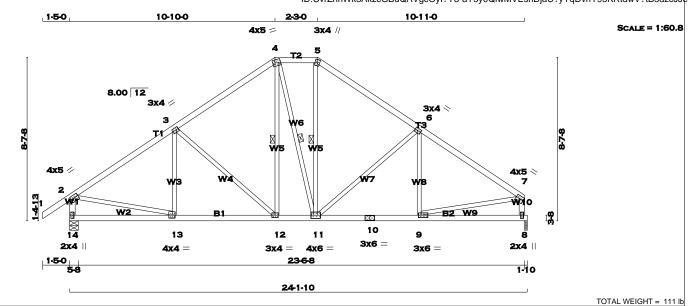




READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

JOB PESCENE PARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY DRWG NO. Page 106 of 159 TW0317-048 TW0317-048 T45 Kott Lumber Uxbridge, Stouffville, ON, TW

Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:53 2017 Page ID:CvrZnhWk3AkzcGBuQAVgcOyI?Y8-aY5y0QlwMVEshDjaU?yYqDvhY95KRtdwV?tB5dzcJJe



LUMBER				
N. L. G. A. R	ULES			
CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 5	2x4	DRY	No.2	SPF
5 - 7	2x4	DRY	No.2	SPF
14 - 2	2x4	DRY	No.2	SPF
8 - 7	2x4	DRY	No.2	SPF
14 - 10	2x4	DRY	No.2	SPF
10 - 8	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PL/	PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Υ	Χ	
2	TMVW-t	MT20	4.0	5.0	1.75	2.00	
3	TMWW-t	MT20	3.0	4.0	1.50	1.50	
4	TTWW-m	MT20	4.0	5.0	1.75	1.50	
5	TTW+m	MT20	3.0	4.0	2.00	1.25	
6	TMWW-t	MT20	3.0	4.0	1.50	1.50	
7	TMVW-t	MT20	4.0	5.0	1.75	Edge	
8	BMV1+p	MT20	2.0	4.0		-	
9	BMWW-t	MT20	3.0	6.0	1.50	1.75	
10	BS-t	MT20	3.0	6.0			
11	BMWWW-t	MT20	4.0	6.0			
12	BMWW-t	MT20	3.0	4.0			
13	BMWW-t	MT20	4.0	4.0	1.75	1.75	
1/	RM\/1±n	MT20	2.0	4.0			

Edge - INDICATES REFERENCE CORNER OF PLATE

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

IMENSIONS, SUPPORTS	AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY
BUILDING DESIGNER	
EADINGS	

BEARINGS											
	FACTORED		MAXIMU	M FACT	INPUT	REQRD					
	GROSS R	EACTION	GROSS	REACTIO	BRG	BRG					
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX				
14	1254	0	1334	371	-566	5-8	5-8				
8	1138	0	1206	0	-500	1-10	1-10				

PROVIDE ANCHORAGE AT BEARING JOINT 14 FOR 566 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 8 FOR 500 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES

PROVIDE FOR 371 LBS FACTORED HORIZONTAL REACTION AT JOINT 14

UNI	ACTURED RI	EACTIONS					
	1ST LCASE	MAX./N	IIN. COMPO	NENT REACTION	ONS		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
14	877	629 / 0	0/0	0/0	199 / -564	249 / 0	0/0
8	799	559 / 0	0/0	0/0	170 / -512	240 / 0	0/0
HOF	RIZONTAL RE	ACTIONS					
14		0/0	0/0	0/0	265 / -257	0/0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 14, 8

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.23 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 4-12, 4-11, 5-11. DBS = 20-0-0 . CBF =

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING TOTAL LOAD CASES: (11)

CHORDS					WEBS				
MAX	K. FACTORED	FACTOR	RED				MAX. FACTO	RED	
MEMB.	FORCE	VERT. LO.	AD LC1	MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PL	F) (	CSI (LC)			(LBS)	CSI (LC)	
FR-TO		FROM	TO		LENGTH	FR-TO			
1- 2	0/32	-77.3	-77.3	0.12 (1)		13- 3		0.04 (4)	
2-3	-1355 / 592	-77.3	-77.3	0.48 (7)	5.25	3-12	-443 / 416	0.44 (3)	
3- 4	-1069 / 591	-77.3		0.48(7)		12- 4	-227 / 345	0.10 (7)	
4- 5	-876 / 576	-77.3		0.13 (8)		4-11	-155 / 155	0.08 (5)	
	-1077 / 593	-77.3		0.48(8)		11- 5	-188 / 338	0.09 (7)	
	-1360 / 588	-77.3		0.49(8)		11-6		0.45 (4)	
14- 2	-1294 / 592	0.0	0.0	0.13(1)	7.14	9- 6	-104 / 153	0.04(3)	
8- 7	-1166 / 527	0.0	0.0	0.11(1)	7.42	2-13	-349 / 1154	0.25 (1)	
						9- 7	-378 / 1165	0.25 (1)	
14-13	-350 / 349	-17.5		0.13 (11					
13-12	-542 / 1217	-17.5	-17.5	0.24(1)	6.25				
12-11	-232 / 888	-17.5	-17.5	0.18(1)	6.25				
11-10	-351 / 1146	-17.5	-17.5	0.24(1)	6.25				
10-9	<u>-3</u> 51/1 <u>146</u>	-17.5	-17.5	0.24(1)	6.25				
9-8	- / 2				) 10.00				
WIND	DAD PP	S R	D 01	M R EF	RENCE V	ELOCIT	Y PRESSURE	OF ( 9.0) PS	F,

(40-06) IN-S. T.E. RE. CHECHT ABUYE GRADE AND USING EXTERNAL CEDIT VED COEFFICIENTS, CPCg, BASED ON THE (MAIN WIND FORCE RESIS INCESTING TERMAL)

WIN READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

INCTOYSTEMONTEMINATON
MAY BE LOCATED ON EAST (MART-1209, X2011) AY

17-4978 **BUILDING DIVISION**  **DESIGN CRITERIA** 

SPECIFIED LOADS: LL = 23.3 DL = 3.0 LL = 0.0 DL = 7.0 AD = 33.3 TOP CH. PSF TOTAL LOAD

### SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

[M][F

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09 - TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.80") CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04") ALLOWABLE DEFL.(TL) = L/360 (0.80") CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.49 (6-7:8), BC=0.24 (12-13:1), WB=0.45 (6-11:4), SSI=0.18 (6-7:8)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

### NAIL VALUES

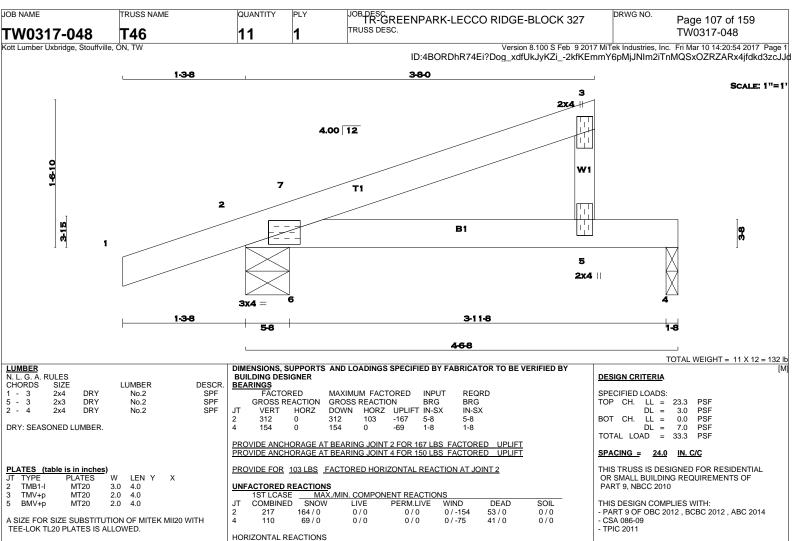
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (9) (INPUT = 0.90) JSI METAL= 0.46 (2) (INPUT = 1.00)





HORIZONTAL REACTIONS 0/0 0/0 74 / 0 0 /0 0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 2, 4

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

<u>LOADING</u> TOTAL LOAD CASES: (11)

CHORDS					WEBS					
MAX.	FACTORED	FACTOR	RED				MAX. FACTO	DRED		
MEMB.	FORCE	VERT. LO.	AD LC1	1 MAX	MAX.	MEMB.	FORCE	MAX		
	(LBS)	(PL	F) (	CSI (LC)	<b>UNBRAC</b>		(LBS)	CSI (LC)		
FR-TO		FROM	TO		LENGTH	FR-TO				
1- 2	0 / 15	-77.3	-77.3	0.10(1)	10.00	6- 7	-46 / 124	0.00(1)		
2-7	-74 / 0	-77.3	-77.3	0.07(1)	6.25					
7-3	-41 / 5	-77.3	-77.3	0.16(1)	6.25					
5-3	-134 / 100	0.0	0.0	0.04 (5)	7.81					
2-6	-14 / 29	-17.5	-17.5	0.08 (1)	6.25					
6- 5	-14 / 29			0.18(1)	6.25					
5- 4	0/0			0.17(1)	10.00					

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF ( 9.0) PSE AT WIND LOAD APPLIED IS DERIVED FROM REPERSING VELOCITY FRESSURE OF (3.9), 25 AT (40-0.0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19") CALCULATED VERT. DEFL.(LL) = L/999 (0.03") ALLOWABLE DEFL.(TL)= L/360 (0.19") CALCULATED VERT. DEFL.(TL) = L/ 978 (0.06")

CSI: TC=0.16 (3-7:1), BC=0.18 (5-6:1), WB=0.00 (6-7:1) , SSI=0.12 (4-5:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

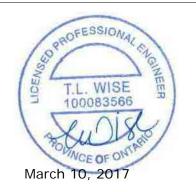
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (2) (INPUT = 0.90 ) JSI METAL= 0.05 (2) (INPUT = 1.00 )





READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

JOB NAME TRUSS NAME

Kott Lumber Uxbridge, Stouffville, ON, TW

T47

TW0317-048

QUANTITY PLY 6

JOB PESC TR-GREENPARK-LECCO RIDGE-BLOCK 327 TRUSS DESC.

DRWG NO.

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SCALE = 1:17.3

Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:54 2017 Page ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-2kfKEmmY6pMjJNIm2iTnMQSyJZSZARa4jfdkd3zcJJd

1-3-8 2x4 | 6.00 12 3x4 / WЭ wi В1 5 3x4 = 1.3-8 5-8

TOTAL WEIGHT = 6 X 13 = 77 lb

LUMBER				
N. L. G. A. R	ULES			
CHORDS	SIZE		LUMBER	DESCR
1 - 3	2x4	DRY	No.2	SPF
5 - 3	2x3	DRY	No.2	SPF
6 - 2	2x4	DRY	No.2	SPF
6 - 4	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEAS				

### PLATES (table is in inches)

JΤ	TYPE	PLATES	W	LEN	Υ	X	
2	TMVW-t	MT20	3.0	4.0	1.50	1.25	
3	TMV+p	MT20	2.0	4.0			
5	BMVW-t	MT20	3.0	4.0			
6	BMV1+p	MT20	2.0	4.0			

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

### DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEA	BEARINGS												
	FACTORED		MAXIMUM FACTORED			INPUT	REQRD						
GROSS REACTION			GROSS REACTION			BRG	BRG						
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX						
6	256	0	256	154	-116	5-8	5-8						
4	101	0	109	0	-62	1-8	1-8						

PROVIDE ANCHORAGE AT BEARING JOINT 6 FOR 150 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 4 FOR 150 LBS FACTORED UPLIFT

## PROVIDE FOR 154 LBS FACTORED HORIZONTAL REACTION AT JOINT 6

## UNFACTORED REACTIONS

	1ST LCASE	MAX./N	MIN. COMPO							
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL			
6	178	136 / 0	0/0	0/0	0 / -109	41 / 0	0/0			
4	72	43 / 0	0/0	0/0	20 / -63	29 / 0	0/0			
1101	DIZONITAL DE	ACTIONS								
HOF	RIZONTAL RE	ACTIONS								
6		0/0	0/0	0 / 0	110 / -49	0/0	0 /0			

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 6, 4

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (12)

CHC	CHORDS				WEBS				
MAX.	FACTORED	FACTO	RED				MAX. FACTO	RED	
MEMB.	FORCE	VERT. LO	AD LC	1 MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PL	.F) (	CSI (LC)	UNBRAC	2	(LBS)	CSI (LC)	
FR-TO		FROM	TO		LENGTH	FR-TO			
1- 2	0 / 23	-77.3	-77.3	0.10(1)	10.00	2- 5	-16 / 121	0.02 (6)	
2-3	-55 / 32	-77.3	-77.3	0.08 (1)	6.25				
5-3	-103 / 79	0.0	0.0	0.07 (7)	7.81				
6- 2	-207 / 108	0.0	0.0	0.02(1)	7.81				
6- 5	-136 / 59	-17.5	-17.5	0.11 (1)	6.25				
5- 4	0/0	-17.5	-17.5	0.11(1)	10.00				

## CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {440-0}, FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.

## **DESIGN CRITERIA**

SPECIFIED LOADS:									
TOP	CH.	LL	=	23.3	PSF				
		DL	=	3.0	PSF				
BOT	CH.	LL	=	0.0	PSF				

DL = 7.0 PSF TOTAL LOAD = 33.3 PSF

### SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014 - CSA 086-09 TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.19") CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.10 (1-2:1) , BC=0.11 (5-6:1) , WB=0.02 (2-5:6) , SSI=0.08 (4-5:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (2) (INPUT = 0.90 ) JSI METAL= 0.05 (5) (INPUT = 1.00 )





READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

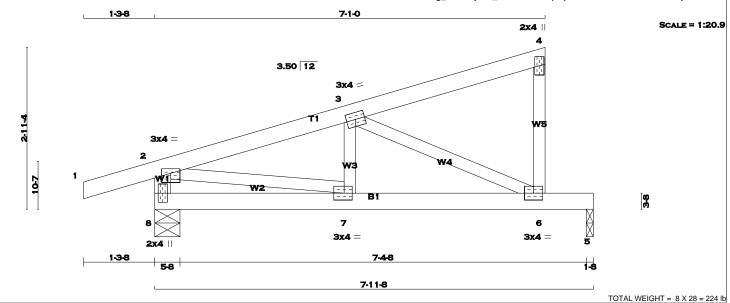
JOB NAME TRUSS NAME QUANTITY PLY TW0317-048 T48 8

JOB PR-GREENPARK-LECCO RIDGE-BLOCK 327

DRWG NO.

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LUMBER				
N. L. G. A. F	RULES			
CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
6 - 4	2x3	DRY	No.2	SPF
8 - 2	2x4	DRY	No.2	SPF
8 - 5	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

Kott Lumber Uxbridge, Stouffville, ON, TW

### PLATES (table is in inches)

TYPE	PLATES	W	LEN	Υ	X
TMVW-p	MT20	3.0	4.0	1.00	2.00
TMWW-t	MT20	3.0	4.0		
TMV+p	MT20	2.0	4.0		
BMVW-t	MT20	3.0	4.0		
BMWW-t	MT20	3.0	4.0	1.50	1.75
BMV1+p	MT20	2.0	4.0		
	TMVW-p TMWW-t TMV+p BMVW-t BMWW-t	TMVW-p MT20 TMWW-t MT20 TMV+p MT20 BMVW-t MT20 BMWW-t MT20	TMVW-p MT20 3.0 TMVW-t MT20 3.0 TMV+p MT20 2.0 BMVW-t MT20 3.0 BMWW-t MT20 3.0	TMVW-p MT20 3.0 4.0 TMWW-t MT20 3.0 4.0 TMV+p MT20 2.0 4.0 BMVW-t MT20 3.0 4.0 BMWW-t MT20 3.0 4.0	TMVW-p         MT20         3.0         4.0         1.00           TMWV+t         MT20         3.0         4.0         1.00           TMV+p         MT20         2.0         4.0         8           BMVW-t         MT20         3.0         4.0         1.50           BMWW-t         MT20         3.0         4.0         1.50

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

### DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEAI	RINGS						
	FACTOR	ED	MAXIMUM FACTORED			INPUT	REQRD
	<b>GROSS RE</b>	ACTION	GROSS F	REACTIO	BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	<b>UPLIFT</b>	IN-SX	IN-SX
8	477	0	477	193	-249	5-8	5-8
5	313	0	313	0	-161	1-8	1-8

PROVIDE ANCHORAGE AT BEARING JOINT 8 FOR 249 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 5 FOR 161 LBS FACTORED UPLIFT

PROVIDE FOR 193 LBS FACTORED HORIZONTAL REACTION AT JOINT 8

### UNFACTORED REACTIONS

	1ST LCASE	MAX./	MIN. COMPOR				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
8	332	245 / 0	0/0	0/0	0 / -234	87 / 0	0/0
5	221	147 / 0	0/0	0/0	0 / -163	75 / 0	0/0
HOR 8	IZONTAL REA	ACTIONS 0/0	0/0	0/0	138 / 0	0/0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 8, 5

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

<u>LOADING</u> TOTAL LOAD CASES: (11)

CHORDS					WEBS				
MAX.	FACTORED	FACTO	RED				MAX. FACTO	DRED	
MEMB.	FORCE	VERT. LC	DAD LC	1 MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PL	_F) (	CSI (LC)	UNBRAC	2	(LBS)	CSI (LC)	
FR-TO		FROM	TO		LENGTH	FR-TO			
1- 2	0 / 14	-77.3	-77.3	0.10(1)	10.00	7-3	0 / 91	0.03 (11)	
2-3	-581 / 301	-77.3	-77.3	0.16 (5)	6.25	3-6	-619 / 384	0.16(1)	
3- 4	-50 / 36	-77.3	-77.3	0.12 (5)	6.25	2-7	-251 / 578	0.13(1)	
6- 4	-108 / 79	0.0	0.0	0.10 (5)	7.81				
8- 2	-467 / 272	0.0	0.0	0.05 (1)	7.81				
8- 7	-180 / 0	-17.5	-17.5	0.10(1)	6.25				
7-6	-298 / 564	-17.5	-17.5	0.45 (1)	6.25				
6- 5	0/0	-17.5	-17.5	0.36(1)	10.00				

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CPCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

### **DESIGN CRITERIA**

TOTAL LOAD

SPECIFIED LOADS: LL = 23.3 DL = 3.0 LL = 0.0 DL = 7.0 AD = 33.3 CH. PSF PSF

### SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

PSF

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014 - CSA 086-09 TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.27")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.27") CALCULATED VERT. DEFL.(TL) = L/ 973 (0.10")

CSI: TC=0.16 (2-3:5), BC=0.45 (6-7:1), WB=0.16 (3-6:1) , SSI=0.24 (5-6:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

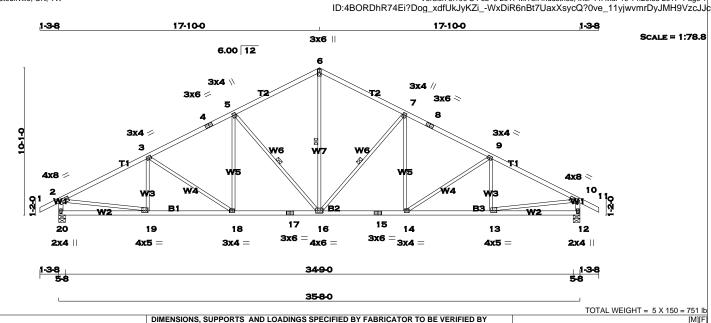
JSI GRIP= 0.86 (6) (INPUT = 0.90 ) JSI METAL= 0.21 (7) (INPUT = 1.00 )





READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

JOB PESC PREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY DRWG NO. Page 110 of 159 TRUSS DESC TW0317-048 TW0317-048 T49 Kott Lumber Uxbridge, Stouffville, ON, TW Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:55 2017 Page



LUMBER				
N. L. G. A. F	RULES			
CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 6	2x4	DRY	No.2	SPF
6 - 8	2x4	DRY	No.2	SPF
8 - 11	2x4	DRY	No.2	SPF
20 - 2	2x4	DRY	No.2	SPF
12 - 10	2x4	DRY	No.2	SPF
20 - 17	2x4	DRY	No.2	SPF
17 - 15	2x4	DRY	No.2	SPF
15 - 12	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
FYCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)									
TYPE	PLATES	W	LEN	Υ	Χ				
TMVW-t	MT20	4.0	8.0	1.75	3.00				
TMWW-t	MT20	3.0	4.0	1.50	1.75				
TS-t	MT20	3.0	6.0						
TMWW+t	MT20	3.0	4.0	1.75	0.75				
TTW+p	MT20	3.0	6.0						
TMWW+t	MT20	3.0	4.0	1.75	0.75				
TS-t	MT20	3.0	6.0						
TMWW-t	MT20	3.0	4.0	1.50	1.75				
TMVW-t	MT20	4.0	8.0	1.75	3.00				
BMV1+p	MT20	2.0	4.0	2.25	1.00				
BMWW-t	MT20	4.0	5.0	1.50	1.50				
BMWW-t	MT20	3.0	4.0						
BS-t	MT20	3.0	6.0						
BMWWW-t	MT20	4.0	6.0	1.75	3.00				
BS-t	MT20	3.0	6.0						
BMWW-t	MT20	3.0	4.0						
BMWW-t	MT20	4.0	5.0	1.50	1.50				
BMV1+p	MT20	2.0	4.0	2.25	1.00				
	TYPE TMVW-t TMVW-t TS-t TMWW+t TTW+p TMWW+t TS-t TMVW-t BMVW-t BMVW-t BMWW-t BS-t BMWW-t BS-t BMWW-t BMWW-t BMWW-t BMWW-t BMWW-t BMWW-t BMWW-t BMWW-t BMWW-t	TYPE PLATES TMVW-t MT20 TMVW-t MT20 TS-t MT20 TMWW+t MT20 TTW+p MT20 TTW+p MT20 TMVW-t MT20 TMVW-t MT20 TMVW-t MT20 TMVW-t MT20 BMVH-t MT20 BMW-t MT20 BMWW-t MT20	TYPE         PLATES         W           TMVW-t         MT20         4.0           TMWW-t         MT20         3.0           TS-t         MT20         3.0           TS+t         MT20         3.0           TTW+p         MT20         3.0           TMWW+t         MT20         3.0           TMWW-t         MT20         3.0           TMWW-t         MT20         4.0           BMV1+p         MT20         2.0           BMWW-t         MT20         3.0           BS-t         MT20         3.0           BMWW-t         MT20         4.0           BS-t         MT20         3.0           BMWW-t         MT20         3.0	TYPE         PLATES         W         LEN           TMWV+t         MT20         4.0         8.0           TMWV+t         MT20         3.0         4.0           TS+t         MT20         3.0         6.0           TTMWW+t         MT20         3.0         4.0           TTW+p         MT20         3.0         6.0           TMWW+t         MT20         3.0         6.0           TMWW-t         MT20         3.0         4.0           TMWV+t         MT20         3.0         4.0           BMV1+p         MT20         2.0         4.0           BMWW+t         MT20         3.0         6.0           BS+t         MT20         3.0         6.0           BS-t         MT20         3.0         6.0           BS-t         MT20         3.0         6.0           BS-t         MT20         3.0         6.0           BS-t         MT20         3.0         6.0           BSWW-t         MT20         3.0         6.0           BMWW-t         MT20         3.0         6.0           BMWW-t         MT20         3.0         6.0	TYPE				

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

PLATES (table is in inches)											
	TYPE	PLATES	w	LEN	Υ	Χ					
2	TMVW-t	MT20	4.0	8.0	1.75	3.00					
3	TMWW-t	MT20	3.0	4.0	1.50	1.75					
4	TS-t	MT20	3.0	6.0							
5	TMWW+t	MT20	3.0	4.0	1.75	0.75					
6	TTW+p	MT20	3.0	6.0							
7	TMWW+t	MT20	3.0	4.0	1.75	0.75					
8	TS-t	MT20	3.0	6.0							
9	TMWW-t	MT20	3.0	4.0	1.50	1.75					
10	TMVW-t	MT20	4.0	8.0	1.75	3.00					
12	BMV1+p	MT20	2.0	4.0	2.25	1.00					
13	BMWW-t	MT20	4.0	5.0	1.50	1.50					
14	BMWW-t	MT20	3.0	4.0							
15	BS-t	MT20	3.0	6.0							
16	BMWWW-t	MT20	4.0	6.0	1.75	3.00					
17	BS-t	MT20	3.0	6.0							
18	BMWW-t	MT20	3.0	4.0							
19	RM\/\/\/_t	MT20	4 0	5.0	1.50	1.50					

BUILDING DESIGNER BEARINGS

	FACTOR	ED	MAXIMUN	M FACTO	INPUT	REQRD	
	<b>GROSS RE</b>	REACTION GROSS REACTION E				BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
20	1796	0	1832	251	-804	5-8	5-8
12	1796	0	1832	0	-804	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 20 FOR 804 LBS FACTORED PROVIDE ANCHORAGE AT BEARING JOINT 12 FOR 804 LBS FACTORED

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES

PROVIDE FOR 251 LBS FACTORED HORIZONTAL REACTION AT JOINT 20

UNF	<u>ACTORED RE</u>	EACTIONS					
	1ST LCASE	MAX.	MIN. COMPON	ENT REACTION	ONS		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
20	1258	894 / 0	0/0	0/0	90 / -809	365 / 0	0/0
12	1258	894 / 0	0/0	0/0	90 / -809	365 / 0	0/0
HOR 20	IZONTAL REA	ACTIONS 0/0	0/0	0/0	179 / -179	0/0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 20, 12

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.95 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.03 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 6-16, 7-16, 5-16. DBS = 20-0-0 . CBF =

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING TOTAL LOAD CASES: (11)

	ORDS X. FACTORED	FACTO	DED			WE	B S MAX. FACTO	)PED
MEMB.		VERT. LC		1 ΜΔΥ	MAX.	МЕМВ.		MAX
IVILIVID.	(LBS)			CSI (LC)			(LBS)	CSI (LC)
FR-TO	(LDO)	FROM			LENGTH		(LDO)	001 (L0)
1- 2	0 / 23	-77.3		0.10(1)	10.00	16- 6	-568 / 1200	0.38 (8)
2- 3	-2569 / 1107			0.49 (7)				0.41 (4)
3- 4	-2290 / 1053			0.48 (7)				0.10 (8)
4- 5	-2290 / 1053			0.48 (7)			-337 / 325	0.31 (4)
5-6	-1783 / 913			0.44 (7)		13- 9	-213 / 209	0.06 (1)
6- 7	-1783 / 913	-77.3	-77.3	0.44 (8)	4.68	5-16	-752 / 573	0.41 (3)
7-8	-2290 / 1053	-77.3	-77.3	0.48 (8)	4.24	18- 5	-110 / 286	0.10 (7)
8- 9	-2290 / 1053	-77.3	-77.3	0.48 (8)	4.24	3-18	-337 / 325	0.31 (3)
9-10	-2569 / 1107	-77.3	-77.3	0.49 (8)	3.95	19- 3	-213 / 209	0.06 (1)
10-11	0 / 23	-77.3	-77.3	0.10(1)	10.00	2-19	-861 / 2327	0.53 (7)
20- 2	-1786 / 833	0.0		0.18 (1)	6.30	13-10	-861 / 2327	0.53 (8)
12-10	-1786 / 833	0.0	0.0	0.18 (1)	6.30			
20-19	-234 / 268			0.14 (11				
19-18	-1087 / 2365			0.42 (1)				
18-17	-816 / 2085			0.40 (1)	6.25			
17-16	-816 / 2085			0.40 (1)			-	
16-15	-621 / 2044			0.40 (1)			р	ECEIVED
15-14	-621 / 2044			0.40 (1)				
14-13	-836 / 2297			0.42 (1)	6.25		TOW	'N OF MILTOI
13-12	-9 / 18	-17.5	-17.5	0.14 (11	10.00		2.4	ND 00 0047
							IVI <i>I</i>	AR 29, 2017
								17-4978

**DESIGN CRITERIA** 

SPECIFIED LOADS: LL = DL = LL = DL = AD = CH. 3.0 PSF 7.0 PSF

### SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

33.3 PSF

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014

- CSA 086-09 TPIC 2011

TOTAL LOAD

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.19") CALCULATED VERT. DEFL.(LL) = L/999 (0.14") ALLOWABLE DEFL.(TL)= L/360 (1.19") CALCULATED VERT. DEFL.(TL) = L/ 999 (0.26")

CSI: TC=0.49 (9-10:8) , BC=0.42 (13-14:1) , WB=0.53 (10-13:8) , SSI=0.19 (9-10:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

**BUILDING DIVISION** 

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (16) (INPUT = 0.90 ) JSI METAL= 0.69 (13) (INPUT = 1.00 )



CONTINUED ON PAGE 2

TRUSS NAME
TWO317-048
T49

TRUSS DESC.

TR-GREENPARK-LECCO RIDGE-BLOCK 327
TRUS DESC.

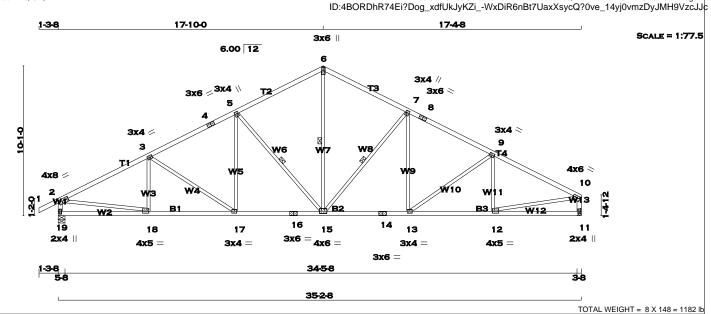
Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:55 2017 Page 2 ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-WxDiR6nBt7UaxXsycQ?0ve\_11yjwvmrDyJMH9VzcJJc

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.0) PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CCQ.6 BASED ON THE (MAIN WIND PORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2), BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

JOB PESC PREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY DRWG NO. Page 112 of 159 TRUSS DESC TW0317-048 TW0317-048 T50 8 Kott Lumber Uxbridge, Stouffville, ON, TW Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:55 2017 Page

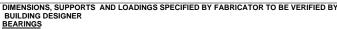


LUMBER				
N. L. G. A. R	ULES			
CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 6	2x4	DRY	No.2	SPF
6 - 8	2x4	DRY	No.2	SPF
8 - 10	2x4	DRY	No.2	SPF
19 - 2	2x4	DRY	No.2	SPF
11 - 10	2x4	DRY	No.2	SPF
19 - 16	2x4	DRY	No.2	SPF
16 - 14	2x4	DRY	No.2	SPF
14 - 11	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

	PLATES (table is in inches)									
JT	TYPE	PLATES	W	LEN	Υ	X				
2	TMVW-t	MT20	4.0	8.0	1.75	3.00				
3	TMWW-t	MT20	3.0	4.0	1.50	1.75				
4	TS-t	MT20	3.0	6.0						
5	TMWW+t	MT20	3.0	4.0	1.75	0.75				
6	TTW+p	MT20	3.0	6.0						
7	TMWW+t	MT20	3.0	4.0	1.75	0.75				
8	TS-t	MT20	3.0	6.0						
9	TMWW-t	MT20	3.0	4.0	1.50	1.75				
10	TMVW-t	MT20	4.0	6.0	1.50	2.75				
11	BMV1+p	MT20	2.0	4.0						
12	BMWW-t	MT20	4.0	5.0	1.50	1.75				
13	BMWW-t	MT20	3.0	4.0						
14	BS-t	MT20	3.0	6.0						
15	BMWWW-t	MT20	4.0	6.0	1.75	3.00				
16	BS-t	MT20	3.0	6.0						
17	BMWW-t	MT20	3.0	4.0						
18	BMWW-t	MT20	4.0	5.0	1.50	1.50				
19	BMV1+p	MT20	2.0	4.0	2.25	1.00				

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.



	DEARINGS											
	FACTO	RED	MAXIMU	IM FACT	ORED	INPUT	REQRD					
	GROSS R	EACTION	GROSS	REACTIO	N	BRG	BRG					
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX					
19	1775	0	1811	269	-796	5-8	5-8					
11	1670	0	1708	0	-728	HANGER	BY OTHERS					
						MIN. SEAT	Γ SIZE: 3-8					

PROVIDE ANCHORAGE AT BEARING JOINT 19 FOR 796 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 11 FOR 728 LBS FACTORED UPLIFT

ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES. SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 269 LBS FACTORED HORIZONTAL REACTION AT JOINT 19

UNE	ACTORED RE	ACTIONS								
	1ST LCASE	MAX./I	MIN. COMPO	NENT REACTION	ONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL			
19	1243	883 / 0	0/0	0/0	90 / -800	360 / 0	0/0			
11	1172	820 / 0	0/0	0/0	95 / -746	352 / 0	0/0			
HOR	HORIZONTAL REACTIONS									
19		0/0	0/0	0/0	192 / -157	0/0	0 /0			

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 19

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.98 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 6.02 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 5-15, 6-15, 7-15. DBS = 20-0-0 . CBF = 89 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

**LOADING** TOTAL LOAD CASES: (11)

СН	ORDS					W E	BS	
MAX	K. FACTORED	FACTO	RED				MAX. FACT	ORED
MEMB.	FORCE	VERT. LC	AD LC	1 MAX	MAX.	MEMB.	FORCE	MAX
	(LBS)	(PI	_F) (	CSI (LC)	UNBRAG	0	(LBS)	CSI (LC)
FR-TO	( - /		ΤΌ	( -,	LENGTH	FR-TO	( -/	( -/
1- 2	0 / 23	-77.3	-77.3	0.10(1)	10.00	18- 3	-208 / 207	0.05 (1)
2-3	-2531 / 1093	-77.3		0.48 (7)		3-17	-341 / 327	0.32 (3)
3- 4	-2247 / 1037	-77.3	-77.3	0.48 (7)		17- 5	-110 / 289	0.10 (7)
4- 5	-2247 / 1037	-77.3	-77.3	0.48 (7)	4.28	5-15	-755 / 573	0.41 (3)
5- 6	-1737 / 892	-77.3		0.44 (7)		15- 6	-556 / 1165	0.37 (7)
6- 7	-1738 / 898	-77.3		0.42 (8)	4.76	15- 7	-693 / 543	0.37 (4)
7-8	-2191 / 1010	-77.3	-77.3	0.46 (8)	4.34	13- 7	-94 / 240	0.08 (8)
8- 9	-2191 / 1010	-77.3	-77.3	0.46 (8)	4.34	13- 9	-249 / 280	0.23 (4)
9-10	-2384 / 1023	-77.3	-77.3	0.46 (8)	4.12	12-9	-281 / 235	0.08 (1)
19- 2	-1764 / 825	0.0	0.0	0.17 (1)	6.33	2-18	-849 / 2293	0.53 (7)
11-10	-1663 / 756	0.0	0.0	0.17 (1)	6.48	12-10	-825 / 2171	0.49 (8)
				. ,				. ,
19-18	-253 / 226	-17.5	-17.5	0.14 (11	) 6.25			
18-17	-1093 / 2326	-17.5	-17.5	0.42(1)	6.02			
17-16	-820 / 2042	-17.5	-17.5	0.39 (1)	6.25			
16-15	-820 / 2042	-17.5	-17.5	0.39 (1)	6.25			
15-14	-608 / 1952	-17.5	-17.5	0.38(1)	6.25			EOEN/E
14-13	-608 / 1952	-17.5	-17.5	0.38(1)	6.25		K	ECEIVE
13-12	-790 / 2126	-17.5	-17.5	0.40(1)	6.25		TOV	VN OF MIL
12-11	-11 / 23	-17.5	-17.5	0.14 (11	) 6.25		.0.	
				`			M	AR 29, 20
								0, _0

ED LTON 17 17-4978 **BUILDING DIVISION** 

# **DESIGN CRITERIA**

SPECIFIED LOADS: LL = 23.3 DL = 3.0 LL = 0.0 DL = 7.0 AD = 33.3 CH. PSF PSF TOTAL LOAD PSF

### SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012, BCBC 2012, ABC 2014 - CSA 086-09 - TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (1.17") CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.48 (2-3:7) , BC=0.42 (17-18:1) , WB=0.53 (2-18:7) , SSI=0.19 (2-3:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (12) (INPUT = 0.90 ) JSI METAL= 0.68 (18) (INPUT = 1.00 )



TRUSS NAME
TSO
8
1
TRUSS DESC.

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Nersion 8.100 S Feb 9.2017 MiTek Industries, Inc. Fri Mar 10 14:20:55 2017 Page 2
ID:4BORDhR74Ei?Dog\_xdfUkJykZi\_-WxDiR6nBt7UaxXsycQ?0ve\_14yj0vmzDyJMH9VzcJJc

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.0) PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, Opcg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM,INTERNAL WIND PRESSURE IS BASED ON DER SASED ON THE SASED O



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

JOB PESC TR-GREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY DRWG NO Page 114 of 159 TRUSS DESC TW0317-048 TW0317-048 T51 Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:56 2017 Page 1 Kott Lumber Uxbridge, Stouffville, ON, TW ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-\_7m4eSnpeQcRYhR9A7WFRrX78Mzxe7?NBz6rixzcJJb 1-3-8 1-11-5 5-4-0 149-11 1690 1-3-8 SCALE = 1:79.8 4x5 || 8 3x5 \\ 3x4 // 3x6 = 3x6 > 6 10 4x4 < 3x4 < 6.00 12 8x14 = 11 W9 W1 wio 5x8 = 4x8 = 4x8 > W1 12 2 Ţ.Į W14 B **B2 B**1 W15 18 <sup>17</sup> 20 21 19 16 15 24 23 22 14 **4**x6 = 5x6 = 4x10 = 3x8 3x4 = 4x6 = 2x4 || 2x4 || **4**x5 = 4x4 || 3x6 = 37-11-0 38-10-0 TOTAL WEIGHT = 162 lb LUMBER N. L. G. A. RULES CHORDS SIZE DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER **DESIGN CRITERIA** LUMBER DESCR BEARINGS FACTORED 3 DRY No.2 SPF SPF MAXIMUM FACTORED INPLIT REQRD SPECIFIED LOADS: LL = DL = LL = DL = AD = No.2 GROSS REACTION GROSS REACTION CH. HORZ UPLIFT IN-SX 3.0 6 2x4 DRY No 2 SPF VFRT HOR7 DOWN IN-SX PSF 2x4 No.2 24 14 237 5-8 10 2x4 DRY No.2 SPF 1980 -842 5-8 5-8 7.0 PSF 10 -24 -DRY DRY SPF 13 No.2 TOTAL LOAD 33.3 PSF 2x4 PROVIDE ANCHORAGE AT BEARING JOINT 24 FOR 912 LBS FACTORED No.2 14 -24 -20 -12 20 17 2x4 DRY No.2 SPF SPACING = 24.0 IN. C/C 1650F 1.5E 1650F 1.5E SPF 2x4 NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES DRY 2x4 17 -14 DRY 1650F 1.5E SPF LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12 PROVIDE FOR 237 LBS FACTORED HORIZONTAL REACTION AT JOINT 24 ALL WEBS 2x3 DRY No.2 SPF THIS TRUSS IS DESIGNED FOR RESIDENTIAL UNFACTORED REACTIONS OR SMALL BUILDING REQUIREMENTS OF DRY: SEASONED LUMBER PART 9, NBCC 2010 THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09 - TPIC 2011

PLATES (table is in inches)									
JT	TYPE	PLATES	W	LEN	Υ	Χ			
2	TMVW-p	MT20	4.0	8.0	1.25	3.50			
3	TTWW-m	MT20	8.0	14.0	Edge	6.25			
4	TTWW-m	MT20	5.0	8.0	2.75	4.00			
5	TMWW-t	MT20	4.0	4.0	1.75	1.25			
6	TS-t	MT20	3.0	6.0					
7	TMWW+t	MT20	3.0	5.0	2.25	0.75			
8	TTW+p	MT20	4.0	5.0	2.25	2.00			
9	TMWW+t	MT20	3.0	4.0	1.75	0.75			
10	TS-t	MT20	3.0	6.0					
11	TMWW-t	MT20	3.0	4.0	1.50	1.75			
12	TMVW-t	MT20	4.0	8.0	1.75	3.00			
14	BMV1+p	MT20	2.0	4.0	2.25	1.00			
15	BMWW-t	MT20	4.0	6.0	1.75	1.75			
16	BMWW-t	MT20	3.0	4.0					
17	BS-t	MT20	3.0	6.0					
18	BMWWW-t	MT20	5.0	6.0	2.25				
19	BMWW+t	MT20	4.0	4.0	1.50	1.75			
20	BS-t	MT20	3.0	8.0					
21	BMWW-t	MT20	4.0	6.0	1.50				
22	BMWW-t	MT20	4.0		1.75	3.25			
23	BMWW-t	MT20	4.0	5.0	1.50	1.50			
24	BMV1+p	MT20	2.0	4.0	2.25	1.00			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.



	1ST LCASE	MAX./	<u>MIN. COMPON</u>	<u>IENT REACTIO</u>	DNS		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
24	1364	967 / 0	0/0	0/0	121 / -906	396 / 0	0/0
14	1364	967 / 0	0/0	0/0	84 / -856	396 / 0	0/0
HOR 24	IZONTAL REA	ACTIONS 0/0	0/0	0/0	169 / -169	0/0	0 /0

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED

ALLOWABLE DEFL.(LL) = L/360 (1.29") CALCULATED VERT. DEFL.(LL) = L/ 999 (0.42") ALLOWABLE DEFL.(TL) = L/360 (1.29") CALCULATED VERT. DEFL.(TL) = L/638 (0.73")

CSI: TC=0.78 (3-4:1), BC=0.82 (21-22:1),

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN

618 354 1667 822 2284 1656 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.90 (3) (INPUT = 0.90 ) JSI METAL= 0.97 (20) (INPUT = 1.00 )

**CONTINUED ON PAGE 2** 

TRUSS MANUFACTURING PLANT

NAIL VALUES

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10 COMPANION LIVE LOAD FACTOR = 0.50

WB=0.92 (3-22:1), SSI=0.18 (11-12:1)

ROOF LIVE LOAD

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 24, 14

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.34 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 4.69 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 4-21, DBS = 6-0-0, CBF = 89 LBS 1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 4-21. DBS = 6-0-0 . CBF = 89 LBS. 1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 7/2 LENGTH OF 5-19. DBS = 10-0-0 . CBF = 83 LBS. 1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 7-18. DBS = 12-0-0 . CBF = 93 LBS. 1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 8-18. DBS = 18-0-0 . CBF = 89 LBS. 1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 9-18. DBS = 20-0-0 . CBF = 78 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL (S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING TOTAL LOAD CASES: (11)

		` '						
СН	ORDS					W E	BS	
MAX	(. FACTORED	FACTORE	D				MAX. FACTO	RED
MEMB.	FORCE	VERT. LOAI	D LC1	1 MAX	MAX.	MEMB	. FORCE	MAX
	(LBS)	(PLF)	(	CSI (LC)	UNBRAC		(LBS)	CSI (LC)
FR-TO		FROM TO	0		LENGTH	FR-TO	) ' '	
1- 2	0 / 23	-77.3 -	77.3	0.10(1)	10.00	23-3	-677 / 363	0.10 (1)
2- 3	-2248 / 1021	-77.3 -	77.3	0.19(7)	4.56	3-22	-1795 / 4201	0.92 (1)
	-5934 / 2619						-1387 / 699	0.20 (1)
	-4146 / 1825	-77.3 -	77.3	0.48(7)	3.20	4-21	-2445 / 1167	0.53 (1)
	-3007 / 1379	-77.3 -					-364 / 984	0.22 (1)
	-3007 / 1379	-77.3 -					-1387 / 772	0.39 (1)
7-8	-2183 / 1064	-77.3 -	77.3	0.37(7)	4.41	19- 7	-438 / 980	0.37 (7)
	-2191 / 1087						-1293 / 789	0.59 (3)
9-10	-2623 / 1145	-77.3 -	77.3	0.45 (8)		18-8	-757 / 1608	0.44 (7)
	-2623 / 1145	-77.3 -				18- 9	-666 / 523	0.32 (4)
	-2811 / 1170							0.07 (8)
12-13		-77.3 -					-238 / 279	0.19 (4)
	-2031 / 931			0.20(1)		15-11	-274 / 221	0.07 (1)
14-12	-1935 / 868	0.0	0.0	0.19 (1)	6.09	2-23	-8 <mark>96 / 2168</mark>	0.48 (1)
						15-12	-9: 4 / 2548	E <b>Č</b> ĚÍVED
	-220 / 254							
23-22	-1026 / 2019	-17.5 -					TOW	N OF MILTON
22-21	-2759 / 6061	-17.5 -						
21-20	-1666 / 3770	-17.5 -					M/	AR 29, 2017
20-19	-1666 / 3770	-17.5 -						47 4070
19-18	-1084 / 2723	-17.5 -						17-4978
18-17	-773 / 2353						51	NING BIN ((G) GN)
	-773 / 2353						BUILL	DING DIVISION
16-15	-897 / 2515	-17.5 -	17.5	0.33(1)	6.25			

JOB DESC TR-GREENPARK-LECCO RIDGE-BLOCK 327 TRUSS DESC. JOB NAME TRUSS NAME QUANTITY PLY DRWG NO. Page 115 of 159 1 TW0317-048 TW0317-048 T51 Kott Lumber Uxbridge, Stouffville, ON, TW Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:56 2017 Page 2 ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-\_7m4eSnpeQcRYhR9A7WFRrX78Mzxe7?NBz6rixzcJJb

LOADING TOTAL LOAD CASES: (11)

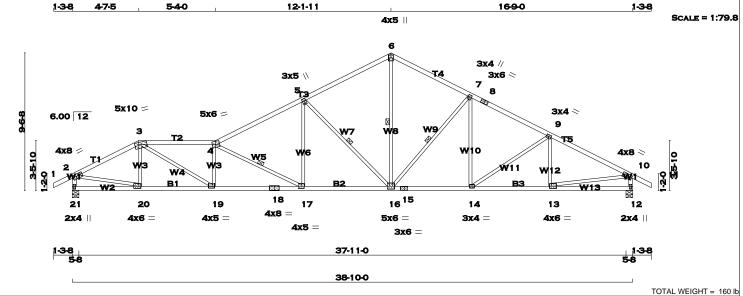
C H O R D S
MAX. FACTORED

EMB. FORCE
(LBS)
(LBS)
(PLF)
(CSI (LC)
(PLF)
(CSI (LC)
(PLF)
(CSI (LC)
(PLF)
(CSI (LC)
(PLF)
(FROM TO
(FROM TO WEBS MAX. FACTORED
. FORCE MAX
(LBS) CSI (LC) MEMB. FR-TO 15-14

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF ( 9.0) PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CPCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

POBPES GREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY DRWG NO. Page 116 of 159 TRUSS DESC. TW0317-048 TW0317-048 T52 Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:57 2017 Page Kott Lumber Uxbridge, Stouffville, ON, TW ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-SJKTsooRPkklAq0Ljq1U\_34LKmJwNdcWQdrOEOzcJJa

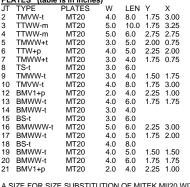


LUMBER				
N. L. G. A. R	ULES			
CHORDS	SIZE		LUMBER	DESCR.
1 - 3	2x4	DRY	No.2	SPF
3 - 4	2x4	DRY	No.2	SPF
4 - 6	2x4	DRY	No.2	SPF
6 - 8	2x4	DRY	No.2	SPF
8 - 11	2x4	DRY	No.2	SPF
21 - 2	2x4	DRY	No.2	SPF
12 - 10	2x4	DRY	No.2	SPF
21 - 18	2x4	DRY	No.2	SPF
18 - 15	2x4	DRY	No.2	SPF
15 - 12	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				-

DRY: SEASONED LUMBER

PL.	PLATES (table is in inches)										
JT	TYPE	PLATES	W	LEN	Υ	X					
2	TMVW-t	MT20	4.0	8.0	1.75	3.00					
3	TTWW-m	MT20	5.0	10.0	1.75	3.25					
4	TTWW-m	MT20	5.0	6.0	2.75	2.75					
5	TMWW+t	MT20	3.0	5.0	2.00	0.75					
6	TTW+p	MT20	4.0	5.0	2.25	2.00					
7	TMWW+t	MT20	3.0	4.0	1.75	0.75					
8	TS-t	MT20	3.0	6.0							
9	TMWW-t	MT20	3.0	4.0	1.50	1.75					
10	TMVW-t	MT20	4.0	8.0	1.75	3.00					
12	BMV1+p	MT20	2.0	4.0	2.25	1.00					
13	BMWW-t	MT20	4.0	6.0	1.75	1.75					
14	BMWW-t	MT20	3.0	4.0							
15	BS-t	MT20	3.0	6.0							
16	BMWWW-t	MT20	5.0	6.0	2.25	3.00					
17	BMWW-t	MT20	4.0	5.0	1.75	2.00					
18	BS-t	MT20	4.0	8.0							
19	BMWW-t	MT20	4.0	5.0	1.50	1.50					
20	BMWW-t	MT20	4.0	6.0	1.75	1.75					
21	BMV1+p	MT20	2.0	4.0	2.25	1.00					

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.



T.L. WISE 100083566 NCE OF ON March 10, 2017

DIME	NSIONS, S	UPPORTS	ANDIO	ADINGS	SPECIFIE	FD BY F	ABRICATOR	TO BE VERIFIED BY
	LDING DES				oo		,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	. 0 22 12.1 122 21
		IGNER						
BEA	RINGS							
	FACTO	RFD	MAXIMU	M FACT	ORFD	INPUT	REORD	
	GROSS R	EACTION	GROSS	REACTION	JN	BRG	BRG	
JT	VFRT	HOR7	DOWN	HOR7	UPLIFT.	IN-SX	IN-SX	
•					O. L	0/1	071	

-842

5-8

5-8

PROVIDE ANCHORAGE AT BEARING JOINT 21 FOR 912 LBS\_FACTORED\_UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES

PROVIDE FOR 237 LBS FACTORED HORIZONTAL REACTION AT JOINT 21

1980

UNF	UNFACTORED REACTIONS											
	1ST LCASE	MAX./I	MIN. COMPO	NENT REACTION	SNC							
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL					
21	1364	967 / 0	0/0	0/0	121 / -906	396 / 0	0/0					
12	1364	967 / 0	0/0	0/0	84 / -856	396 / 0	0/0					
HORIZONTAL REACTIONS												
21		0/0	0/0	0/0	169 / -169	0/0	0 /0					

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 21, 12

12

1947

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.00 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 4.79 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 4-17. DBS = 8-0-0 . CBF = 81 LBS 1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 5-16. DBS = 10-0-0 . CBF = 85 LBS. 1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 6-16. DBS = 18-0-0 . CBF = 87 LBS. 1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 7-16. DBS = 20-0-0 . CBF = 78 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING TOTAL LOAD CASES: (11)

	O R D S C. FACTORED FORCE	VERT. LO	AD LC1		MAX.	MEMB		MAX
FR-TO 1- 2	(LBS) 0 / 23	FROM	TO	0.10 (1)	LENGTH	FR-TO	(LBS) -341 / 239	CSI (LC) 0.07 (1)
2- 3 3- 4	-2740 / 1228 -4366 / 1959	-77.3 -77.3	-77.3 -77.3	0.42 (7) 0.55 (1)	3.99 3.00	3-19 19- 4	-924 / 2249 -1085 / 557	0.57 (7) 0.22 (1)
5- 6	-3237 / 1446 -2192 / 1051 -2192 / 1086	-77.3	-77.3	0.57 (7)	4.20	17- 5	-1677 / 860 -321 / 892 -1425 / 868	0.53 (1) 0.22 (7) 0.71 (3)
7- 8 8- 9	-2623 / 1145 -2623 / 1145	-77.3 -77.3	-77.3 -77.3	0.45 (8) 0.45 (8)	4.02 4.02	16- 6 16- 7	-732 / 1583 -665 / 525	0.43 (7) 0.32 (4)
10-11 21- 2	-2812 / 1170 0 / 23 -1962 / 935	-77.3 0.0	-77.3 0.0	0.45 (8) 0.10 (1) 0.19 (1)	10.00 6.07	14- 9 13- 9		0.07 (8) 0.19 (4) 0.07 (1)
12-10 21-20	-1936 / 869 -220 / 254	0.0 -17.5		0.19 (1)		2-20 13-10	-983 / 2479 -924 / 2549	0.55 (1) 0.57 (1)
20-19 19-18	-1179 / 2486 -1987 / 4445	-17.5 -17.5	-17.5 -17.5	0.44 (1) 0.77 (1)	5.86 4.79		D	ECEIVED
18-17 17-16 16-15	-1987 / 4445 -1225 / 2958 -774 / 2353	-17.5	-17.5	0.77 (1) 0.53 (1) 0.44 (1)	5.79			N OF MILTON
	-774 / 2353 -897 / 2515 -9 / 18	-17.5 -17.5	-17.5 -17.5	0.44 (1) 0.45 (1)	6.25 6.25		M	AR 29, 2017 17-4978
13-12	-9/18	-17.5	-17.5	0.12 (11)	) 10.00			17 7370

**DESIGN CRITERIA** 

SPECIFIED LOADS: LL = DL = LL = DL = AD = CH. 3.0 PSF 7.0 PSF TOTAL LOAD 33.3

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09

- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.29") CALCULATED VERT. DEFL.(LL)= L/ 999 (0.30") ALLOWABLE DEFL.(TL)= L/360 (1.29") CALCULATED VERT. DEFL.(TL)= L/871 (0.54")

CSI: TC=0.63 (4-5:7) , BC=0.77 (17-19:1) , WB=0.71 (5-16:3) , SSI=0.20 (4-5:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

**BUILDING DIVISION** 

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (3) (INPUT = 0.90 ) JSI METAL= 0.87 (18) (INPUT = 1.00 )

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. TR-GREENPARK-LECCO RIDGE-BLOCK 327	DRWG NO. Page 117 of 159
TW0317-048	T52	1	1	TRUSS DESC.	TW0317-048
Kott Lumber Uxbridge, Stouffville	e, ON, TW		1-	Version 8.100 S Feb 9 201	17 MiTek Industries, Inc. Fri Mar 10 14:20:57 2017 Page 2 ooRPkkIAq0Ljq1U_34LKmJwNdcWQdrOEOzcJJi
				ID.4BONDIIN/4EI?D0g_xulox3yxZi_33x130	DIT KIAGOLJQ I O_S4LKIIISWINGCWQQIOLOZCOS
		WIND LOAD AP {40-0-0} FT-IN-5	PLIED IS DERIN	/ED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT E HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK O ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL DIN DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON S IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY	
		COEFFICIENTS WIND PRESSU	S, CpCg, BASED RE IS BASED C	O ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON	
		FROM EAVE.	N}, AND TRUSS	S IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY	



JOB PESC TR-GREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY DRWG NO. Page 118 of 159 TRUSS DESC TW0317-048 TW0317-048 T53 Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:57 2017 Page 1 Kott Lumber Uxbridge, Stouffville, ON, TW ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-SJKTsooRPkklAq0Ljq1U\_34NRmMRNZdWQdrOEOzcJJa 1690 1.3-8 7-3-5 540 9-5-11 1.3-8 SCALE = 1:79.8 4x5 || 3x4 // 3x5 \\ 3x6 < 8 5x8 = 5x5 = 6.00 12 3x4 < 3x4 / 10 W9 W1 iο 4-9-10 4x8 = 4x8 > 11 W14 **B2** BI W15 W2 17 16 19 21 18 15 14 13 23 22 20 3x8 = **5**x6 = 4x6 4x4 = 3x4 = 4x6 = 2x4 || 3x4 2x4 || 4x4 3x6 = 37-11-0 38-10-0 TOTAL WEIGHT = 167 lb DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY **BUILDING DESIGNER DESIGN CRITERIA** SPECIFIED LOADS: CH. PSF 3.0 PSF 7.0 PSF

LUMBER				
N. L. G. A. R	ULES			
CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 5	2x4	DRY	No.2	SPF
5 - 7	2x4	DRY	No.2	SPF
7 - 9	2x4	DRY	No.2	SPF
9 - 12	2x4	DRY	No.2	SPF
23 - 2	2x4	DRY	No.2	SPF
13 - 11	2x4	DRY	No.2	SPF
23 - 19	2x4	DRY	No.2	SPF
19 - 16	2x4	DRY	No.2	SPF
16 - 13	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER

	PLATES (table is in inches)										
JT	TYPE	PLATES	W	LEN	Υ	X					
2	TMVW-t	MT20	4.0	8.0	1.50	3.00					
3	TMWW-t	MT20	3.0	4.0	1.50	1.75					
4	TTWW-m	MT20	5.0	8.0	2.00	3.00					
5	TTWW-m	MT20	5.0	5.0	3.00	2.00					
6	TMWW+t	MT20	3.0	5.0	2.25	0.75					
7	TTW+p	MT20	4.0	5.0	2.25	2.00					
8	TMWW+t	MT20	3.0	4.0	1.75	0.75					
9	TS-t	MT20	3.0	6.0							
10	TMWW-t	MT20	3.0	4.0	1.50	1.75					
11	TMVW-t	MT20	4.0	8.0	1.75	3.00					
13	BMV1+p	MT20	2.0	4.0	2.25	1.00					
14	BMWW-t	MT20	4.0	6.0	1.75	1.75					
15	BMWW-t	MT20	3.0	4.0							
16	BS-t	MT20	3.0	6.0							
17	BMWWW-t	MT20	5.0	6.0	2.25	3.00					
18	BMWW-t	MT20	4.0	4.0	1.50	2.00					
19	BS-t	MT20	3.0	8.0							
20	BMWW-t	MT20	4.0	4.0	1.75	1.50					
21	BMWW-t	MT20	3.0	4.0							
22	BMWW-t	MT20	4.0	6.0	1.75	1.50					
23	BMV1+p	MT20	2.0	4.0	2.25	1.00					

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.



3EA	KINGS						
	FACTOR	ED	MAXIMU	M FACTO	INPUT	REQRD	
	<b>GROSS RE</b>	ACTION	GROSS F	REACTIO	N	BRG	BRG
JΤ	VERT	HORZ	DOWN	HORZ	<b>UPLIFT</b>	IN-SX	IN-SX
23	1947	0	1995	237	-912	5-8	5-8
13	1947	0	1980	0	-842	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 23 FOR 912 LBS FACTORED PROVIDE ANCHORAGE AT BEARING JOINT 13 FOR 842 LBS FACTORED

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES

PROVIDE FOR 237 LBS FACTORED HORIZONTAL REACTION AT JOINT 23

UNF	UNFACTORED REACTIONS											
	1ST LCASE	MAX./	MAX./MIN. COMPONENT REACTIONS									
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL					
23	1364	967 / 0	0/0	0/0	121 / -906	396 / 0	0/0					
13	1364	967 / 0	0/0	0/0	84 / -856	396 / 0	0/0					
HOF	HORIZONTAL REACTIONS											
23		0/0	0/0	0/0	169 / -169	0/0	0 /0					

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 23, 13

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.41 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 5.28 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

- 1 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 6-17. DBS = 12-0-0 . CBF = 93 LBS. 1 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 7-17. DBS = 18-0-0 . CBF = 89 LBS.
- 1 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 8-17. DBS = 20-0-0 . CBF = 78 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING TOTAL LOAD CASES: (11)

СН	ORDS					W E	BS	
	K. FACTORED	FACTO	RED				MAX. FACTO	ORED
MEMB.	FORCE	VERT. LO		1 MAX	MAX.	MEMB.		MAX
	(LBS)				UNBRAG		(LBS)	
FR-TO	( - /	FROM			LENGTH			( -/
1- 2	0 / 23	-77.3	-77.3	0.10(1)	10.00	22- 3	-497 / 286	0.09 (1)
2- 3	-2647 / 1217			0.31 (7)	4.16	3-21	-8 / 152	0.03 (1)
3- 4	-2804 / 1323	-77.3	-77.3	0.32 (7)	4.07	21-4	0 / 61	0.02 (11)
4- 5	-3548 / 1631	-77.3	-77.3	0.49 (7)	3.41	4-20	-530 / 1387	0.45 (7)
5- 6	-2953 / 1353	-77.3	-77.3	0.44 (7)	3.84	20-5	-824 / 416	0.28 (1)
6- 7	-2186 / 1071	-77.3	-77.3	0.40 (7)	4.41	5-18	-1257 / 678	0.96 (3)
7-8	-2191 / 1087	-77.3	-77.3	0.42 (8)	4.34	18- 6	-408 / 943	0.36 (7)
8- 9	-2623 / 1145	-77.3	-77.3	0.45 (8)	4.02	6-17	-1291 / 793	0.59 (3)
9-10	-2623 / 1145	-77.3	-77.3	0.45 (8)	4.02	17- 7	-772 / 1623	0.45 (7)
10-11	-2812 / 1170	-77.3	-77.3	0.45 (8)	3.83	17-8	-666 / 523	0.32 (4)
11-12	0 / 23	-77.3	-77.3	0.10(1)	10.00	15-8	-89 / 227	0.07 (8)
23-2	-1958 / 926	0.0	0.0	0.19(1)	6.08	15-10	-238 / 279	0.19 (4)
13-11	-1936 / 869	0.0	0.0	0.19(1)	6.09	14-10	-274 / 220	0.07 (1)
						2-22	-1011 / 2431	0.53 (1)
	-220 / 254			0.07 (8)	6.25	14-11	-924 / 2548	0.57 (1)
22-21	-1205 / 2428	-17.5	-17.5	0.43 (1)	5.83			
21-20	-1149 / 2536			0.44 (1)	5.93		D	ECEIVED.
20-19	-1560 / 3605	-17.5	-17.5	0.61 (1)	5.28			ECEIVED
19-18	-1560 / 3605			0.61 (1)	5.28		TOW	'N OF MILTON
18-17	-1069 / 2694	-17.5	-17.5	0.48 (1)	6.10			
	-773 / 2353			0.44 (1)	6.25		M	AR 29, 2017
	-773 / 2353			0.44 (1)	6.25			47 4070
15-14	-897 / 2515			0.45 (1)	6.25			17-4978
14-13	-9 / 18	-17.5	-17.5	0.12 (11)	10.00		DI III I	DINIC DIVICION

LL = DL = LL = DL = AD = TOTAL LOAD 33.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09 - TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.29") CALCULATED VERT. DEFL.(LL)= L/ 999 (0.24") ALLOWABLE DEFL.(TL)= L/360 (1.29") CALCULATED VERT. DEFL.(TL)= L/ 999 (0.43")

CSI: TC=0.49 (4-5:7) , BC=0.61 (18-20:1) , WB=0.96 (5-18:3) , SSI=0.18 (10-11:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

**BUILDING DIVISION** 

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (20) (INPUT = 0.90) JSI METAL= 0.96 (19) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB PESC DE ENDA DIVI ECCO DIDOE DI OCIV 227	DRWG NO. Page 110 of 150
TW0317-048	T53	1	1	JOB DESC TR-GREENPARK-LECCO RIDGE-BLOCK 327 TRUSS DESC.	Page 119 of 159 TW0317-048
Kott Lumber Uxbridge, Stouffvill		•	•	Version 8.100 S Feb 9 2017	/ MiTek Industries, Inc. Fri Mar 10 14:20:57 2017 Page
				ID:4BORDhR74Ei?Dog_xdfUkJyKZiSJKTsooR	RPkkIAq0Ljq1U_34NRmMRNZdWQdrOEOzcJ
		WIND LOAD A	PPLIED IS DEF	RIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT	
		{40-0-0} FT-IN-	SX REFEREN	ICE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK	
		WIND PRESSI	JRE IS BASED	RIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT ICE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK ED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL 0 ON DESIGN (CATEGORY 2), BUILDING MAY BE LOCATED ON ISS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY	
		FROM EAVE.	iiv, AND TRO	100 10 DESIGNED TO BE ECCATED AT EEAST (0-0) FT-IN-5X AWAT	



JOB PESC TR-GREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY PLY DRWG NO Page 120 of 159 TRUSS DESC TW0317-048 TW0317-048 T54 Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:58 2017 Page Kott Lumber Uxbridge, Stouffville, ON, TW ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-wWur38p3A2s9o\_bXHYYjXGcYhAkj675feHbymqzcJJZ 1.3-8 9-11-5 6-9-11 1690 1-3-8 SCALE = 1:79.8 4x5 Ⅱ 7 AYA 3x4 // 5x8 = 5x6 < 3x6 < 8 6.00 12 T2 3x4 < 3x4 / 10 W10 3 6-1-10 ÷ 4x8 = 4x8 > 11 WI3 **B**1 **B2** W2 WIA 17 16 15 13 21 20 19 18 14 4x6 = 6x8 = 5x6 = 3x4 = 4x6 = 2x4 || 3x4 = 2x4 || 3x6 = 37-11-0 38-10-0 TOTAL WEIGHT = 169 lb DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER **DESIGN CRITERIA** BEARINGS FACTORED MAXIMUM FACTORED INPLIT REQRD SPECIFIED LOADS: GROSS REACTION GROSS REACTION CH. BRG LL = DL = HORZ UPLIFT IN-SX 3.0 .IT VFRT HOR7 DOWN IN-SX PSF LL = DL = AD = 21 13 237 5-8 1947 1980 -842 5-8 5-8 7.0 **PSF** TOTAL LOAD 33.3 PROVIDE ANCHORAGE AT BEARING JOINT 21 FOR 912 LBS FACTORED SPACING = 24.0 IN. C/C NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES LOADING IN ALL FLAT SECTIONS BASED ON A

LUMBER				
N. L. G. A. R	ULES			
CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 5	2x4	DRY	No.2	SPF
5 - 7	2x4	DRY	No.2	SPF
7 - 9	2x4	DRY	No.2	SPF
9 - 12	2x4	DRY	No.2	SPF
21 - 2	2x4	DRY	No.2	SPF
13 - 11	2x4	DRY	No.2	SPF
21 - 18	2x4	DRY	No.2	SPF
18 - 16	2x4	DRY	No.2	SPF
16 - 13	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PL	PLATES (table is in inches)								
JT	TYPE	PLATES	W	LEN	Υ	X			
2	TMVW-t	MT20	4.0	8.0	1.50	3.00			
3	TMWW-t	MT20	3.0	4.0	1.50	1.75			
4	TTWW-m	MT20	5.0	8.0	2.25	3.75			
5	TTW-h	MT20	5.0	6.0	2.75	3.25			
6	TMWW-t	MT20	4.0	4.0	1.50	1.25			
7	TTW+p	MT20	4.0	5.0	2.25	2.00			
8	TMWW+t	MT20	3.0	4.0	1.75	0.75			
9	TS-t	MT20	3.0	6.0					
10	TMWW-t	MT20	3.0	4.0	1.50	1.75			
11	TMVW-t	MT20	4.0	8.0	1.75	3.00			
13	BMV1+p	MT20	2.0	4.0	2.25	1.00			
14	BMWW-t	MT20	4.0	6.0	1.75	1.75			
15	BMWW-t	MT20	3.0	4.0					
16	BS-t	MT20	3.0	6.0					
17	BMWWW-t	MT20	5.0	6.0	2.00	3.00			
18	BSWWW-I	MT20	6.0	8.0					
19	BMWW-t	MT20	3.0	4.0					
20	BMWW-t	MT20	4.0	6.0	1.75	1.75			
21	BMV1+p	MT20	2.0	4.0	2.25	1.00			

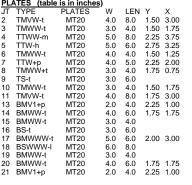
A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

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March 10, 2017



CHORDS MAX. FACTORED МЕМВ. FORCE (LBS) FR-TO

FACTORED VERT. LOAD LC1 MAX (PLF) CSI (LC) FROM TO -77.3 -77 -77.3 -77 -2807 / 1283 -2705 / 1291 -2984 / 1406 -77.3 -77.3 -77.3 -77.3 -77.3 -3363 / 1642 -2194 / 1086 -2622 / 1145 -2622 / 1145 -77.3 -77.3 -2812 / 1170 -77.3 -1953 / 934 0.0 -1936 / 868 0.0 -220 / 254 -17.5 -1248 / 2576 -17.5 -1049 / 2441 -17.5 -17.5 -935 / 2455 -774 / 2352 -774 / 2352

PROVIDE FOR 237 LBS FACTORED HORIZONTAL REACTION AT JOINT 21

	1ST LCASE	MAX.	MAX./MIN. COMPONENT REACTIONS								
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL				
21	1364	967 / 0	0/0	0/0	121 / -906	396 / 0	0/0				
13	1364	967 / 0	0/0	0/0	84 / -856	396 / 0	0/0				
HOR	HORIZONTAL REACTIONS										
21		0/0	0/0	0/0	169 / -169	0/0	0 /0				

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 21, 13

UNFACTORED REACTIONS

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.71 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 5.74 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

- 1 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 5-18. DBS = 8-0-0 . CBF = 89 LBS. - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 6-18, 8-17. DBS = 20-0-0 . CBF = 85 LBS.
- 1 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 6-17. DBS = 12-0-0 . CBF = 86 LBS. 1 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 7-17. DBS = 18-0-0 . CBF = 92 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

WEBS

MEMB.

MAX. FACTORED

FORCE

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING TOTAL LOAD CASES: (11)

SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09 - TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED

ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.29") CALCULATED VERT. DEFL.(LL)= L/ 999 (0.23") ALLOWABLE DEFL.(TL)= L/360 (1.29") CALCULATED VERT. DEFL.(TL)= L/999 (0.43")

CSI: TC=0.46 (4-5:7), BC=0.48 (17-18:1), WB=0.57 (11-14:1), SSI=0.18 (10-11:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (7) (INPUT = 0.90) JSI METAL= 0.65 (16) (INPUT = 1.00)

MAX.

OB NAME TW0317-048	TRUSS NAME	QUANTITY	PLY	JOB DESC TR-GRE TRUSS DESC.	ENPARK-LE	CCO RIDGE-E	BLOCK 327	DRWG NO.	Page 121 of 15 TW0317-048	9
Kott Lumber Uxbridge, Stouffvil			<b>I</b>			Version 8	3.100 S Feb 9 2017	 MiTek Industries, In	c. Fri Mar 10 14:20:58 20	117 Page 2
					ID:4BORDhR	74Ei?Dog_xdfU	kJyKZiwWur38	p3A2s9o_bXHY	YjXGcYhAkj675feHb	ymqzcJJŻ
		WIND LOAD AI (40-0-0) FT-IN- COEFFICIENT WIND PRESSI (OPEN TERRA FROM EAVE.	PPLIED IS DERI SX REFERENC IS, CpCg, BASE JRE IS BASED I NIN}, AND TRUS	VED FROM REFEI E HEIGHT ABOVE D ON THE (MAIN V ON DESIGN (CATE S IS DESIGNED TO	RENCE VELOCITY GRADE AND USIN VIND FORCE RESI GORY 2). BUILDIN D BE LOCATED AT	PRESSURE OF { 9 NG EXTERNAL PEA STING SYSTEM}.IN NG MAY BE LOCAT I LEAST {0-0} FT-IN	.0) PSF AT K ITERNAL ED ON -SX AWAY			
										ļ



POBPES GREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY DRWG NO. Page 122 of 159 TRUSS DESC. TW0317-048 TW0317-048 T55 Kott Lumber Uxbridge, Stouffville, ON, TW Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:59 2017 Page 1 ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-OiSDHTqhxL\_?P8AkrF3y3U9gTa4qrWiptxKVIGzcJJ\ 12-7-5 1-3-8 12.7.5 13-7-7 1-3-8 SCALE = 1:65.2 4x8 = 2x4 || 4x8 = 6 6.00 12 3x4 = 3x4 < 7 W6 W6 4x8 < 4x8 < WЗ W/3 W2 10 15 13 17 16 12 11 2x4 || 3x6 = 3x6 = 2x4 || **4**x6 = **3x4** = **3x8** = 3x4 = **4**x6 = 1-3-8 5-8 37-11-0 38-10-0 TOTAL WEIGHT = DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER **DESIGN CRITERIA** SPECIFIED LOADS: CH. PSF

LUMBER				
N. L. G. A. F	ULES			
CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 6	2x4	DRY	No.2	SPF
6 - 9	2x4	DRY	No.2	SPF
18 - 2	2x4	DRY	No.2	SPF
10 - 8	2x4	DRY	No.2	SPF
18 - 15	2x4	DRY	No.2	SPF
15 - 13	2x4	DRY	No.2	SPF
13 - 10	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PL	PLATES (table is in inches)									
JT	TYPE	PLATES	W	LEN	Υ	Χ				
2	TMVW-t	MT20	4.0	8.0	1.75	3.00				
3	TMWW-t	MT20	3.0	4.0	1.50	1.75				
4	TTWW-m	MT20	4.0	8.0	1.75	3.75				
5	TMW+w	MT20	2.0	4.0						
6	TTWW-m	MT20	4.0	8.0	1.75	3.75				
7	TMWW-t	MT20	3.0	4.0	1.50	1.75				
8	TMVW-t	MT20	4.0	8.0	1.75	3.00				
10	BMV1+p	MT20	2.0	4.0	2.25	1.00				
11	BMWW-t	MT20	4.0	6.0	1.75	1.75				
12	BMWW-t	MT20	3.0	4.0						
13	BS-t	MT20	3.0	6.0						
14	BMWWW-t	MT20	3.0	8.0						
15	BS-t	MT20	3.0	6.0						
16	BMWW-t	MT20	3.0	4.0						
17	BMWW-t	MT20	4.0	6.0	1.75	1.75				
18	BMV1+p	MT20	2.0	4.0	2.25	1.00				

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

T.L. WISE 100083566

100083566

NOEOFON

March 10, 2017

BEARINGS										
FACTORED		MAXIMUM FACTORED			INPUT	REQRD				
GROSS RE	GROSS REACTION			BRG	BRG					
VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX				
1947	0	2006	-189	-964	5-8	5-8				
1947	0	1968	0	-964	5-8	5-8				
	FACTOR GROSS RE VERT 1947	FACTORED GROSS REACTION VERT HORZ 1947 0	FACTORED MAXIMUI GROSS REACTION GROSS F VERT HORZ DOWN 1947 0 2006	FACTORED MAXIMUM FACTOR GROSS REACTION GROSS REACTION VERT HORZ DOWN HORZ 1947 0 2006 -189	FACTORED MAXIMUM FACTORED GROSS REACTION GROSS REACTION VERT HORZ DOWN HORZ UPLIFT 1947 0 2006 -189 -964	FACTORED MAXIMUM FACTORED INPUT GROSS REACTION GROSS REACTION BRG VERT HORZ DOWN HORZ UPLIFT IN-SX 1947 0 2006 -189 -964 5-8				

PROVIDE ANCHORAGE AT BEARING JOINT 18 FOR 964 LBS FACTORED PROVIDE ANCHORAGE AT BEARING JOINT 10 FOR 964 LBS FACTORED

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES

PROVIDE FOR 189 LBS FACTORED HORIZONTAL REACTION AT JOINT 18

### UNFACTORED REACTIONS

	1ST LCASE	MAX./	<u>MIN. COMPON</u>	ONS			
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
18	1364	967 / 0	0/0	0/0	149 / -943	396 / 0	0/0
10	1364	967 / 0	0/0	0/0	54 / -943	396 / 0	0/0
HOR	IZONTAL RE	ACTIONS					
18		0/0	0/0	0/0	135 / -135	0/0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 18, 10

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 5.66 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 4-14, 6-14. DBS = 20-0-0 . CBF = 43

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING TOTAL LOAD CASES: (11)

	O R D S	FACTO	PED			W E	BS MAX. FAC	TORED
MEMB.	FORCE (LBS)	VERT. LO	AD LC		MAX. UNBRAC			E MAX
FR-TO	, ,	FROM	ΤΌ	. ,	LENGTH	FR-TO	) ` ´	` '
1- 2	0 / 23				10.00		-233 / 245	0.06 (1)
2-3	-2895 / 1403			0.65 (7)		3-16		
3- 4	-2563 / 1327			0.64 (7)				
4- 5	-2641 / 1409	-77.3	-77.3	0.62 (7)	3.72	4-14	-399 / 539	
5-6	-2641 / 1409	-77.3	-77.3	0.62 (7)	3.72	14- 5	-681 / 527	0.67 (3)
6- 7	-2538 / 1327	-77.3	-77.3	0.64 (8)	3.91	14- 6	-399 / 571	0.27 (7)
7-8	-2845 / 1404	-77.3	-77.3	0.65 (8)	3.66	12-6	-140 / 353	0.14 (8)
8- 9	0 / 23	-77.3	-77.3	0.10(1)	10.00	12-7	-437 / 395	0.48 (4)
18-2	-1957 / 994	0.0	0.0	0.19(1)	6.08	11- 7	-229 / 245	0.06 (1)
10-8	-1919 / 994	0.0	0.0	0.19(1)	6.10	2-17	-1128 / 2624	4 0.80 (7)
						11-8	-1128 / 2598	8 0.80 (8)
18-17	-171 / 200	-17.5	-17.5	0.16 (11	) 6.25			
17-16	-1284 / 2648	-17.5	-17.5	0.49 (1)	5.66			
16-15	-954 / 2293	-17.5	-17.5	0.44 (1)	6.25			
15-14	95 / 22			(	6.25			
14-13	/ 2:	17	- 5 - 5	0.4 1)	6.25			
13-12	- / 2:			0.4 1)	6.25			
12-11	= 10 / 2	17	- 5	0.4 1)	5.99			DECENTED
11-10	-9/18	٠٠	5	0.10(11	) 10.00			RECEIVED
_		TO	WN OF MILTO					
	READ ALL NO							
WIND	YEAD ALL NO	PRESSU	<b>RAEACIR</b> (2890) <b>(2617) A</b> T					

ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE
IS AN INTEGRAL PART OF THIS DRAWING AS IT

SING SYSTEM HOPERNAL {40-( COE WIN IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

MAY BE LOCATED ON EABITHLED HINGS BAY WASHON

LL = DL = LL = DL = AD = 3.0 PSF 7.0 PSF TOTAL LOAD 33.3

### SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09 - TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.29")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (1.29")
CALCULATED VERT. DEFL.(TL)= L/999 (0.33")

CSI: TC=0.65 (7-8:8), BC=0.49 (11-12:1), WB=0.80 (8-11:8) , SSI=0.26 (5-6:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

### NAIL VALUES

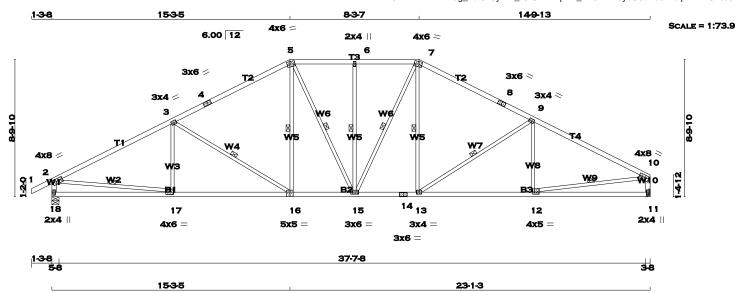
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (4) (INPUT = 0.90) JSI METAL= 0.65 (13) (INPUT = 1.00 )

JOB PR-GREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY DRWG NO. Page 123 of 159 TRUSS DESC TW0317-048 TW0317-048 T56 Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:59 2017 Page Kott Lumber Uxbridge, Stouffville, ON, TW ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-OiSDHTqhxL\_?P8AkrF3y3U9cVa3FralptxKVIGzcJJY



LUMBER				
N. L. G. A. F	RULES			
CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 5	2x4	DRY	No.2	SPF
5 - 7	2x4	DRY	No.2	SPF
7 - 8	2x4	DRY	No.2	SPF
8 - 10	2x4	DRY	No.2	SPF
18 - 2	2x4	DRY	No.2	SPF
11 - 10	2x4	DRY	No.2	SPF
18 - 16	2x4	DRY	No.2	SPF
16 - 14	2x4	DRY	No.2	SPF
14 - 11	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER

PL/	PLATES (table is in inches)									
JT	TYPE	PLATES	W	LEN	Υ	X				
2	TMVW-t	MT20	4.0	8.0	1.75	3.00				
3	TMWW-t	MT20	3.0	4.0	1.50	1.75				
4	TS-t	MT20	3.0	6.0						
5	TTWW-m	MT20	4.0	6.0	1.75	2.25				
6	TMW+w	MT20	2.0	4.0						
7	TTWW-m	MT20	4.0	6.0	1.75	2.25				
8	TS-t	MT20	3.0	6.0						
9	TMWW-t	MT20	3.0	4.0	1.50	1.75				
10	TMVW-t	MT20	4.0	8.0	1.75	Edge				
11	BMV1+p	MT20	2.0	4.0	2.25	1.00				
12	BMWW-t	MT20	4.0	5.0	1.50	1.50				
13	BMWW-t	MT20	3.0	4.0						
14	BS-t	MT20	3.0	6.0						
15	BMWWW-t	MT20	3.0	6.0						
16	BSWW-I	MT20	5.0	5.0	Edge	2.50				
17	BMWW-t	MT20	4.0	6.0	1.75	2.00				
18	BMV1+p	MT20	2.0	4.0	2.25	1.00				

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

SPF		F
SPF		GRO
SPF	JT	VI
SPF	18	19
SPF	11	18
SPF		
SPF		
SPF	PRO	VIDE
SPF	PRO	VIDE
CDE		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS

	FACTORED		MAXIMUM FACTORED			INPUT	REQRD
	GROSS RE	EACTION	GROSS REACTION			BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
18	1925	0	1977	236	-923	5-8	5-8
11	1820	0	1852	0	-856	HANGER E	BY OTHERS
						MIN. SEAT	SIZE: 3-8

ANCHORAGE AT BEARING JOINT 18 FOR 923 LBS FACTORED UPLIFT ANCHORAGE AT BEARING JOINT 11 FOR 856 LBS FACTORED UPLIFT

ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES. SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 236 LBS FACTORED HORIZONTAL REACTION AT JOINT 18

UNF	UNFACTORED REACTIONS										
	1ST LCASE	MAX.	MIN. COMPON	ENT REACTION	ONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD					
18	1349	957 / 0	0/0	0/0	132 / -912	392 / 0					
11	1277	893 / 0	0/0	0/0	79 / -858	384 / 0					
HOF	HORIZONTAL REACTIONS										
18		0/0	0/0	0/0	169 / -139	0/0					

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 18

<u>BRACING</u> TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.28 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 5.67 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 3-16, 5-16, 5-15, 6-15, 7-15, 7-13, 9-13. DBS = 20-0-0 . CBF = 80 LBS.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 2-17. DBS = 10-0-0 . CBF = 79 LBS. 1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 10-12. DBS = 12-0-0 . CBF = 91 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL (S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING TOTAL LOAD CASES: (11)



		` '					
	ORDS				WE		
MA)	X. FACTORED	FACTO	RED			MAX. FACTO	RED
MEMB.	FORCE	VERT. LO	AD LC1 MAX	MAX.	MEMB.	FORCE	MAX
	(LBS)	(PL	.F) CSI (LC)	UNBRAC		(LBS)	CSI (LC)
FR-TO		FROM	TO	LENGTH	FR-TO		
1-2	0 / 23	-77.3	-77.3 0.10 (1)	10.00	17-3	-135 / 232	0.05 (3)
2-3	-2861 / 1336	-77.3	-77.3 0.90 (1)	3.28	3-16	-681 / 554	0.37 (3)
3-4	-2300 / 1168	-77.3	-77.3 0.84 (7)	3.72	16- 5	-219 / 471	0.11 (7)
4- 5	-2300 / 1168	-77.3	-77.3 0.84 (7)	3.72	5-15	-256 / 233	0.15 (8)
5-6	-2126 / 1158	-77.3	-77.3 0.31 (7)	4.54	15-6	-405 / 315	0.19 (3)
6- 7	-2126 / 1158	-77.3	-77.3 0.31 (7)	4.54	15-7	-278 / 299	0.17 (7)
7-8	-2260 / 1154	-77.3	-77.3 0.80 (8)	3.82	13-7	-199 / 425	0.10 (8)
8- 9	-2260 / 1154	-77.3	-77.3 0.80 (8)	3.82	13-9	-577 / 494	0.30 (4)
	-2684 / 1268	-77.3			12- 9	-205 / 260	0.08 (1)
	-1921 / 960	0.0	0.0 0.19 (1)			1047 / 2591	0.57 (1)
11-10	-1797 / 892	0.0	0.0 0.18 (1)		12-10 -	1025 / 2453	0.54 (1)
			,				( )
18-17	-219 / 193	-17.5	-17.5 0.25 (11	) 6.25			
17-16	-1260 / 2624	-17.5					
16-15	-792 / 2051		-17.5 0.40 (1)				
15-14	-683 / 2008		-17.5 0.39 (1)			R	ECEIVED
14-13	-683 / 2008		-17.5 0.39 (1)			TOW	N OF MILTON
13-12	-993 / 2428		-17.5 0.50 (1)			100	IVOI MILION
12-11	-11 / 23		-17.5 0.24 (11			1/1/2	AR 29, 2017
	20		0.2. (	, 5.20		1017	11 20, 2011
							17-4978

**DESIGN CRITERIA** 

SPECIFIED LOADS: LL = DL = LL = DL = AD = CH. PSF 3.0 PSF 7.0 PSF TOTAL LOAD 33.3 PSF

SPACING = 24.0 IN. C/C

THIS DESIGN COMPLIES WITH:

SOIL

0/0 0/0

0 /0

**BUILDING DIVISION** 

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

TOTAL WEIGHT = 163 lb

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09 - TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.28") CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16") ALLOWABLE DEFL.(TL) = L/360 (1.28") CALCULATED VERT. DEFL.(TL) = L/999 (0.30")

CSI: TC=0.90 (2-3:1), BC=0.53 (16-17:1), WB=0.57 (2-17:1) , SSI=0.26 (2-3:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (12) (INPUT = 0.90 ) JSI METAL= 0.73 (12) (INPUT = 1.00)

OB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC TR-GREENPARK-LECCO RIDGE-BLOCK 327	DRWG NO. Page 124 of 159
TW0317-048	T56	1	1	TRUSS DESC.	TW0317-048
Kott Lumber Uxbridge, Stouffvill	le, ÓN, TW	1		Version 8.100 S Feb 9 2017 N ID:4BORDhR74Ei?Dog_xdfUkJyKZiOiSDH	//dizek Industries, Inc. Fri Mar 10 14:20:59 2017 Page 2 Tohxl 2P8AkrF3v3U9cVa3FraIntxKVIGzc.U
				IB. IBONDING TELL BOG_XGIONOYTEL_ 0100FT	THINE_: TO THE OF COOR TO THE PROTECTION OF THE
		WIND LOAD AF	PPLIED IS DER	RIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT CE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK ED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL ON DESIGN (CATEGORY 2), BUILDING MAY BE LOCATED ON SS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY	
		COEFFICIENT	S, CpCg, BASE	DON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL	
		{OPEN TERRA	AIN}, AND TRU	SS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY	
		T I KOIM E/KVE.			



JOB PESC TR-GREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY PLY DRWG NO Page 125 of 159 TRUSS DESC TW0317-048 TW0317-048 **T57** Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:21:00 2017 Page Kott Lumber Uxbridge, Stouffville, ON, TW ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-tu0bUpqJif6s1llwPzaBchitYzHCawly6b42rjzcJJX 4-7-0 1523 1680 1-3-8 1-11-5 SCALE = 1:79.4 8 3x4 \\ 3x4 // 3x6 / 3x6 < 9 10 4x4 < 3x4 > 11 6.00 12 6x10 = T36 W1 O 5x8 = **T6** 4x8 W12 4x8 < 3 12 WI4 Ŵз **M** 6 WЗ B1 19 16 17 23 22 21 20 15 14 13 3x8 = 2x4 || **5**x8 = **4**x6 = **5x6** = **3x4** = **4**x6 = 2x4 || 4x4 = **4**x5 = 3x6 = 37-7-8 38-4-8 TOTAL WEIGHT = 161 lb DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY LUMBER N. L. G. A. RULES CHORDS SIZE BUILDING DESIGNER **DESIGN CRITERIA** LUMBER DESCR BEARINGS FACTORED 3 DRY No.2 SPF SPF MAXIMUM FACTORED INPLIT REQRD SPECIFIED LOADS: LL = DL = LL = DL = AD = No.2 GROSS REACTION GROSS REACTION BRG CH. HORZ UPLIFT IN-SX 3.0 6 2x4 DRY No 2 SPF VFRT HOR7 DOWN IN-SX PSF 2x4 No.2 23 13 1925 260 5-8 5-8 HANGER BY OTHERS 10 2x4 DRY No.2 SPF 1820 0 1856 -770 7.0 PSF DRY DRY SPF 10 -12 No.2 MIN. SEAT SIZE: 3-8 TOTAL LOAD 33.3 PSF 23 -2x4 No.2 13 -23 -19 -SPF SPF SPF 12 19 PROVIDE ANCHORAGE AT BEARING JOINT 23 FOR 898 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 13 FOR 770 LBS FACTORED UPLIFT 2x4 DRY No.2 SPACING = 24.0 IN. C/C 2x4 No.2 DRY 16 2x4 No.2 16 -13 DRY No.2 SPF ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES. LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12 SHALL BE PROVIDED BY BUILDG. DESIGNER ALL WEBS 2x3 DRY No.2 SPF PROVIDE FOR 260 LBS FACTORED HORIZONTAL REACTION AT JOINT 23 THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF DRY: SEASONED LUMBER PART 9, NBCC 2010

PLATES (table is in inches)									
JT	TYPE	PLATES	W	LEN	Υ	Χ			
2	TMVW-p	MT20	4.0	8.0	1.25	3.50			
3	TTWW-m	MT20	6.0	10.0	1.75	2.50			
4	TTWW-m	MT20	5.0	8.0	2.75	4.00			
5	TMWW-t	MT20	4.0	4.0	1.75	1.50			
6	TS-t	MT20	3.0	6.0					
7	TMWW+t	MT20	3.0	4.0	1.50	0.75			
8	TTW+p	MT20	4.0	5.0					
9	TMWW+t	MT20	3.0	4.0	1.75	0.75			
10	TS-t	MT20	3.0	6.0					
11	TMWW-t	MT20	3.0	4.0	1.50	1.75			
12	TMVW-t	MT20	4.0	8.0	1.75	Edge			
13	BMV1+p	MT20	2.0	4.0	2.25	1.00			
14	BMWW-t	MT20	4.0	6.0	1.75	1.75			
15	BMWW-t	MT20	3.0	4.0					
16	BS-t	MT20	3.0	6.0					
17	BMWWW-t	MT20	5.0	6.0		3.00			
18	BMWW-t	MT20	4.0	4.0	1.75	2.00			
19	BS-t	MT20	3.0	8.0					
20	BMWW-t	MT20	4.0	6.0	2.00	2.25			
21	BMWW-t	MT20	5.0	8.0	2.50	2.50			
22	BMWW-t	MT20	4.0	5.0	1.50	1.50			
23	BMV1+p	MT20	2.0	4.0	2.25	1.00			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.



UNF	UNFACTORED REACTIONS								
	1ST LCASE	MAX.	MIN. COMPON	NENT REACTION	ONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
23	1349	957 / 0	0/0	0/0	118 / -893	392 / 0	0/0		
13	1277	893 / 0	0/0	0/0	92 / -797	384 / 0	0/0		

THIS DESIGN COMPLIES WITH:

- CSA 086-09 - TPIC 2011

NAIL VALUES

ROOF LIVE LOAD

- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED

ALLOWABLE DEFL.(LL) = L/360 (1.28") CALCULATED VERT. DEFL.(LL) = L/ 999 (0.35") ALLOWABLE DEFL.(TL) = L/360 (1.28") CALCULATED VERT. DEFL.(TL) = L/749 (0.61")

CSI: TC=0.56 (3-4:1), BC=0.99 (20-21:1), WB=1.00 (5-18:3), SSI=0.18 (11-12:1)

COMP=1.10 SHEAR=1.10 TENS= 1.10 COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN

JSI METAL= 0.92 (19) (INPUT = 1.00)

618 354 1667 822 2284 1656 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.90 (18) (INPUT = 0.90)

**CONTINUED ON PAGE 2** 

TRUSS MANUFACTURING PLANT

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

HORIZONTAL REACTIONS 0/0 0/0 0/0 186 / -152 0/00/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 23

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.68 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 4.30 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 4-20. DBS = 6-0-0 . CBF = 75 LBS 1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 7-17, DBS = 12-0-0, CBF = 88 LBS - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 8-17, 9-17. DBS = 20-0-0 . CBF = 93

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING TOTAL LOAD CASES: (11)

10.742		(,					
C F	HORDS				WEB	s	
MA	X. FACTORED	FACTO	RED		N.	MAX. FACTO	ORED
МЕМВ			DAD LC1 MAX	MAX.	MEMB.	FORCE	MAX
	(LBS)		F) CSI (LC)			(LBS)	CSI (LC)
FR-TO		FROM		LENGTH		( - /	( - /
1- 2	0 / 23		-77.3 0.10 (1)	10.00	22-3 -	688 / 354	0.10 (1)
2- 3	-2200 / 1003		-77.3 0.18 (7)			583 / 3722	0.82 (1)
3- 4	-5371 / 2374		-77.3 0.56 (1)			400 / 696	0.20 (1)
4- 5	-3905 / 1719		-77.3 0.47 (7)			069 / 1005	0.46 (1)
5- 6	-2875 / 1324		-77.3 0.42 (7)			293 / 832	0.18 (1)
6- 7	-2875 / 1324		-77.3 0.42 (7)			253 / 715	1.00 (3)
7-8	-2094 / 1029	-77.3	-77.3 0.38 (7)	4.47	18-7 -	398 / 892	0.36 (7)
8- 9	-2102 / 1055	-77.3	-77.3 0.41 (8)	4.42	7-17 -1:	224 / 763	0.59 (3)
9-10	-2496 / 1098	-77.3	-77.3 0.44 (8)	4.11	17-8 -	724 / 1525	0.44 (7)
10-11	-2496 / 1098	-77.3	-77.3 0.44 (8)	4.11	17-9 -	625 / 506	0.31 (4)
11-12	-2613 / 1087	-77.3	-77.3 0.44 (8)	3.97	15- 9	-75 / 188	0.06 (8)
23- 2	-1992 / 916	0.0	0.0 0.20 (1)	6.03	15-11 -	164 / 244	0.14 (4)
13-12	-1813 / 797	0.0	0.0 0.18 (1)		14-11 -	342 / 249	0.09 (1)
			, ,			878 / 2122	
23-22	-243 / 217	-17.5	-17.5 0.10 (1)	6.25	14-12 -	8 7 / 2381	EČĚÍVED
22-21	-1033 / 1971	-17.5	-17.5 0.43 (1)	6.12		R	ECEIVED
21-20	-2538 / 5494	-17.5	-17.5 0.99 (1)	4.30		TOW	N OF MILTON
20-19	-1593 / 3550	-17.5	-17.5 0.62 (1)	5.24			
19-18	-1593 / 3550	-17.5	-17.5 0.62 (1)	5.24		MA	AR 29, 2017
18-17	-1051 / 2601	-17.5	-17.5 0.47 (1)	6.13			<i>'</i>
17-16	-742 / 2234	-17.5	-17.5 0.42 (1)	6.25			17-4978
16-15	-742 / 2234	-17.5	-17.5 0.42 (1)	6.25			
15-14	-850 / 2333	-17.5	-17.5 0.42 (1)	6.25		BUILE	DING DIVISION
14-13	-11 / 23	-17.5	-17.5 0.12 (11	1) 6.25		-	

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	RK-LECCO RIDGE-BLOCK 32	7	DRWG NO.	Page 126 of 159
TW0317-048	T57	1	1	TRUSS DESC.	THE PERSON OF	•		TW0317-048
Kott Lumber Uxbridge, Stouffville		l			Version 8.100 S Feb 9 ID:4BORDhR74Ei?Dog_xdfUkJyK	2017 MiT	ek Industries, Inc	. Fri Mar 10 14:21:00 2017 Page 2
					ID:4BORDIR/4EI?D0g_XdIUKJYK	<u> </u>	OpqJilos HiwP	Zabchii i zhCawiyob4zijzcJJ.
		WIND LOAD AF	PPLIED IS DEF	RIVED FROM REFERENCE VE	ELOCITY PRESSURE OF { 9.0} PSF AT			
		{40-0-0} FT-IN- COEFFICIENT	SX REFEREN S, CpCg, BAS	CE HEIGHT ABOVE GRADE <i>F</i> ED ON THE {MAIN WIND FOR	ELOCITY PRESSURE OF { 9.0} PSF AT IND USING EXTERNAL PEAK CE RESISTING SYSTEM).INTERNAL BUILDING MAY BE LOCATED ON ATED AT LEAST {0-0} FT-IN-SX AWAY			
		WIND PRESSU (OPEN TERRA	JRE IS BASED IN}, AND TRU	ON DESIGN (CATEGORY 2). SS IS DESIGNED TO BE LOC	BUILDING MAY BE LOCATED ON ATED AT LEAST (0-0) FT-IN-SX AWAY			
		FROM EAVE.	*		, ,			
				) T T				
		A	MI	TT	RECEIVED.			
					RECEIVED TOWN OF MILTON			

MAR 29, 2017 17-4978 **BUILDING DIVISION** 

JOB NAME

JOB PR-GREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY DRWG NO. Page 127 of 159 TW0317-048 TW0317-048 T58 Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:21:00 2017 Page Kott Lumber Uxbridge, Stouffville, ON, TW ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-tu0bUpqJif6s1llwPzaBchirHzLla\_My6b42rjzcJJX 12-6-3 16-8-0 1.38 4-7-0 4.7.5 SCALE = 1:79.4 6 3x4 // 3x5 \\ 3x6 < 3x4 > 6x8 = 5x6 = 6.00 12 **T**5 W 0 4x8 < 4x8 / 10 Wi2 W13 15<sup>14</sup> 19 18 16 13 12 2x4 || 2x4 || **4**x6 = 4x5 = **5x8** = **5**x6 = 3x4 = **4**x6 = 3x6 = 37-7-8 3848

LUMBER				
N. L. G. A. R	ULES			
CHORDS	SIZE		LUMBER	DESCR.
1 - 3	2x4	DRY	No.2	SPF
3 - 4	2x4	DRY	No.2	SPF
4 - 6	2x4	DRY	No.2	SPF
6 - 8	2x4	DRY	No.2	SPF
8 - 10	2x4	DRY	No.2	SPF
19 - 2	2x4	DRY	No.2	SPF
11 - 10	2x4	DRY	No.2	SPF
19 - 16	2x4	DRY	No.2	SPF
16 - 14	2x4	DRY	No.2	SPF
14 - 11	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER

PLATES (table is in inches)									
JT	TYPE	PLATES	W	LEN	Υ	Χ			
2	TMVW-t	MT20	4.0	8.0	1.75	3.00			
3	TTWW-m	MT20	6.0	8.0	2.25	2.75			
4	TTWW-m	MT20	5.0	6.0	2.75	2.75			
5	TMWW+t	MT20	3.0	5.0	2.00	0.75			
6	TTW+p	MT20	4.0	5.0					
7	TMWW+t	MT20	3.0	4.0	1.75	0.75			
8	TS-t	MT20	3.0	6.0					
9	TMWW-t	MT20	3.0	4.0	1.50	1.75			
10	TMVW-t	MT20	4.0	8.0	1.75	Edge			
11	BMV1+p	MT20	2.0	4.0	2.25	1.00			
12	BMWW-t	MT20	4.0	6.0	1.75	1.75			
13	BMWW-t	MT20	3.0	4.0					
14	BS-t	MT20	3.0	6.0					
15	BMWWW-t	MT20	5.0	6.0	2.25	3.00			
16	BSWW-I	MT20	5.0	8.0	3.00	4.00			
17	BMWW-t	MT20	4.0	5.0	1.50	1.75			
18	BMWW-t	MT20	4.0	6.0	1.75	1.75			
19	BMV1+p	MT20	2.0	4.0	2.25	1.00			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED I	ΒY
BUILDING DESIGNER	
BEARINGS	

DEA	KINGS						
	FACTOR	MAXIMUI	M FACTO	ORED	INPUT	REQRD	
	GROSS RE	ACTION	GROSS F	REACTIO	N	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
19	1925	0	1972	260	-898	5-8	5-8
11	1820	0	1856	0	-770	HANGER E	BY OTHERS
						MIN. SEAT	SIZE: 3-8

PROVIDE ANCHORAGE AT BEARING JOINT 19 FOR 898 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 11 FOR 770 LBS FACTORED UPLIFT

ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 260 LBS FACTORED HORIZONTAL REACTION AT JOINT 19

UNFACTORED REACTIONS	

	1ST LCASE	MAX./	MAX./MIN. COMPONENT REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL			
19	1349	957 / 0	0/0	0/0	118 / -893	392 / 0	0/0			
11	1277	893 / 0	0/0	0/0	92 / -797	384 / 0	0/0			
HOR	IZONTAL REA	ACTIONS								
19		0/0	0/0	0/0	186 / -152	0/0	0 /0			

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 19

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.23 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 4.89 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 4-16, 5-15. DBS = 10-0-0 . CBF = 89 LBS.

1-2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 6-15, 7-15. DBS = 20-0-0 . CBF = 92 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL (S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING TOTAL LOAD CASES: (11)

-11 / 23

12-11

СН	ORDS					WE	BS	
MAX	X. FACTORED	FACTO	RED				MAX. FACTO	ORED
MEMB.	FORCE	VERT. LO	AD LC	1 MAX	MAX.	MEMB.	FORCE	MAX
	(LBS)	(PL	.F) (	CSI (LC)	<b>UNBRAC</b>	0	(LBS)	CSI (LC)
FR-TO		FROM	TO		LENGTH	FR-TO		
1- 2	0 / 23			0.10(1)			-344 / 230	0.07 (1)
	-2700 / 1207			0.42 (7)				0.45 (1)
3- 4	-4075 / 1833							0.22 (1)
4- 5	-3097 / 1385					4-16	-1481 / 777	0.49 (1)
5- 6	-2103 / 1015						-270 / 790	0.19 (7)
6- 7	-2102 / 1054			0.41 (8)		5-15	-1350 / 840	0.71 (3)
7-8	-2496 / 1099			0.44 (8)		15- 6	-698 / 1499	0.43 (7)
8- 9	-2496 / 1099	-77.3	-77.3	0.44 (8)	4.11	15- 7	-624 / 508	0.31 (4)
9-10	-2613 / 1087	-77.3	-77.3	0.44 (8)	3.97	13- 7	-77 / 187	0.06 (8)
19- 2	-1937 / 922	0.0	0.0	0.19(1)	6.10	13- 9	-165 / 244	0.14 (4)
11-10	-1813 / 797	0.0	0.0	0.18(1)	6.26	12-9	-341 / 250	0.09 (1)
						2-18	-963 / 2442	0.54 (1)
19-18	-243 / 217	-17.5	-17.5	0.08 (11)	6.25	12-10	-887 / 2381	0.53 (1)
18-17	-1184 / 2445	-17.5	-17.5	0.42 (1)	5.85			
17-16	-1885 / 4150			0.73 (1)			D	ECEN/ED
16-15	-1191 / 2828	-17.5	-17.5	0.52(1)	5.84		K	ECEIVED
15-14	-742 / 2234			0.43 (1)			TOW	/N OF MILTON
14-13	-742 / 2234	-17.5	-17.5	0.43(1)	6.25			
13-12	-850 / 2333	-17.5	-17.5	0.42 (1)	6.25		M	AR 29, 2017

-17.5 0.12 (11)

17-4978 **BUILDING DIVISION**  **DESIGN CRITERIA** 

SPECIFIED LOADS: LL = DL = LL = DL = AD = CH. PSF 3.0 PSF 7.0 PSF TOTAL LOAD 33.3 PSF

### SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

TOTAL WEIGHT = 158 lb

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09

- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.28") CALCULATED VERT. DEFL.(LL)= L/ 999 (0.26") ALLOWABLE DEFL.(TL)= L/360 (1.28") CALCULATED VERT. DEFL.(TL)= L/964 (0.48")

CSI: TC=0.65 (4-5:7) , BC=0.73 (16-17:1) , WB=0.71 (5-15:3) , SSI=0.21 (4-5:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

### NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (10) (INPUT = 0.90) JSI METAL= 0.84 (16) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESCRIPTION AND LECCO PIDGE DI OCK 227	DRWG NO. Page 128 of 150
TW0317-048	T58	1	1	JOB DESC.  JOB DESC.	Page 128 of 159 TW0317-048
Kott Lumber Uxbridge, Stouffville		<u> </u>	•	Version 8.100 S Feb  9 2017	 MiTek Industries, Inc. Fri Mar 10 14:21:01 2017 Page 2
				ID:4BORDhR74Ei?Dog_xdfUkJyKZiL5azi9	ryTzEjfSK6yg5Q8vE01NhXJRb6LFpcN9zcJJV
		WIND LOAD AF	PPLIED IS DER	IVED FROM REFERENCE VELOCITY PRESSURE OF ( 9.0) PSF AT DE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK	
		{40-0-0} FT-IN- COEFFICIENT	SX REFERENC S, CpCg, BASE	CE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK ED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL	
		WIND PRESSI (OPEN TERRA	JRE IS BASED AIN}, AND TRUS	D ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON SS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY	
		FROM EAVE.			



JOB PR-GREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY DRWG NO. Page 129 of 159 TW0317-048 TW0317-048 T59 Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:21:01 2017 Page 1 Kott Lumber Uxbridge, Stouffville, ON, TW ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-L5azi9ryTzEjfSK6yg5Q8vE3zNjlJOE6LFpcN9zcJJW 9-10-3 16-8-0 1-3-8 7.3.5 47-0 SCALE = 1:79.4 3x4 \\ 3x4 // 3x6 < 8 5x8 = 5x5 = 6.00 12 3x4 > 10 3x4 < W9 W1 io **T**5 49-10 12 4x8 W 4x8 < Wi14 W 6 W15 18 17 15 22 19 16 21 20 14 13 2x4 || **4**x6 = **3x4** = **4**x4 = 3x8 **5**x6 = **3x4** = **4**x6 = 2x4 || **4**x4 = **3x6** = 37-7-8

LUMBER				
N. L. G. A. R	ULES			
CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 5	2x4	DRY	No.2	SPF
5 - 7	2x4	DRY	No.2	SPF
7 - 9	2x4	DRY	No.2	SPF
9 - 11	2x4	DRY	No.2	SPF
22 - 2	2x4	DRY	No.2	SPF
12 - 11	2x4	DRY	No.2	SPF
22 - 18	2x4	DRY	No.2	SPF
18 - 15	2x4	DRY	No.2	SPF
15 - 12	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
FXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)									
JT.	TYPE	PLATES	w	LEN	Υ	Х			
2	TMVW-t	MT20	4.0	8.0	1.50	3.00			
3	TMWW-t	MT20	3.0	4.0	1.50	1.75			
4	TTWW-m	MT20	5.0	8.0	2.00	3.25			
5	TTWW-m	MT20	5.0	5.0	3.00	2.25			
6	TMWW+t	MT20	3.0	4.0	1.50	0.75			
7	TTW+p	MT20	4.0	5.0	2.25	2.00			
8	TMWW+t	MT20	3.0	4.0	1.75	0.75			
9	TS-t	MT20	3.0	6.0					
10	TMWW-t	MT20	3.0	4.0	1.50	1.75			
11	TMVW-t	MT20	4.0	8.0	1.75	Edge			
12	BMV1+p	MT20	2.0	4.0	2.25	1.00			
13	BMWW-t	MT20	4.0	6.0	1.75	1.75			
14	BMWW-t	MT20	3.0	4.0					
15	BS-t	MT20	3.0	6.0					
16	BMWWW-t	MT20	5.0	6.0	2.25	3.00			
17	BMWW-t	MT20	4.0	4.0	1.75	2.00			
18	BS-t	MT20	3.0	8.0					
19	BMWW-t	MT20	4.0	4.0	1.75	1.50			
20	BMWW-t	MT20	3.0	4.0					
21	BMWW-t	MT20	4.0	6.0	1.75	1.50			
22	BMV1+p	MT20	2.0	4.0	2.25	1.00			
	•								

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.



DIMENSIONS, SUPPORTS	AND LOADINGS SPECIFIED	BY FABRICATOR TO	BE VERIFIED BY
BUILDING DESIGNER			

3848

BEA	RINGS						
	FACTO	RED	MAXIMU	M FACT	INPUT	REQRD	
	GROSS R	EACTION	GROSS	REACTIO	N	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
22	1925	0	1972	260	-898	5-8	5-8
12	1820	0	1856	0	-770	HANGER E	SIZE: 3-8
						WIII T. OLAI	JIZE. 3-0

PROVIDE ANCHORAGE AT BEARING JOINT 22 FOR 898 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 12 FOR 770 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 260 LBS FACTORED HORIZONTAL REACTION AT JOINT 22

UNFACTORED REACT	<u>IONS</u>	

	1ST LCASE	MAX.	MAX./MIN. COMPONENT REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL			
22	1349	957 / 0	0/0	0/0	118 / -893	392 / 0	0/0			
12	1277	893 / 0	0/0	0/0	92 / -797	384 / 0	0/0			
HOF	RIZONTAL RE	ACTIONS								
22		0/0	0/0	0/0	186 / -152	0/0	0 /0			

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 22

D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.59 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 5.35 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 6-16. DBS = 12-0-0 . CBF = 88 LBS. 1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 7-16. DBS = 18-0-0 . CBF = 85 LBS. 1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 8-16. DBS = 20-0-0 . CBF = 73 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING TOTAL LOAD CASES: (11)

	ORDS					WE	BS		
	X. FACTORED						MAX. F		
MEMB.	FORCE	VERT. LO						RCE	MAX
	(LBS)	(PL	.F) (	CSI (LC)	UNBRAG	0	(LB	S)	CSI (LC)
FR-TO		FROM	TO		LENGTH	I FR-TO			
1-2	0 / 23	-77.3	-77.3	0.10(1)	10.00	21-3	-487 / 2	84	0.09 (1)
2- 3	-2613 / 1196	-77.3	-77.3	0.30 (7)	4.19	3-20	-9 / 1	42	0.03 (1)
3- 4	-2760 / 1299	-77.3	-77.3	0.32 (7)	4.10	20-4	-6 / 5	3	0.02 (11)
4- 5	-3373 / 1556	-77.3	-77.3	0.41 (7)	3.59	4-19	-493 / 1	297	0.33 (7)
5- 6	-2836 / 1303	-77.3	-77.3	0.46 (7)	3.89	19- 5	-845 / 4	16	0.28 (1)
6- 7	-2097 / 1036	-77.3	-77.3	0.42 (7)	4.46	5-17	-1139 / 6	30	0.92 (3)
7-8	-2102 / 1055	-77.3	-77.3	0.41 (8)	4.42	17-6	-362 / 8	53	0.33 (7)
8- 9	-2496 / 1098	-77.3	-77.3	0.44 (8)	4.11	6-16	-1228 / 7	69	0.59 (3)
9-10	-2496 / 1098	-77.3	-77.3	0.44 (8)	4.11	16- 7	-738 / 1	538	0.45 (7)
10-11	-2613 / 1087	-77.3	-77.3	0.44 (8)	3.97	16-8	-625 / 5	06	0.31 (4)
22- 2	-1936 / 912	0.0	0.0	0.19(1)	6.10	14-8	-75 / 1	88	0.06 (8)
12-11	-1813 / 797	0.0	0.0	0.18(1)	6.26	14-10	-164 / 2	44	0.14 (4)
						13-10	-342 / 2	49	0.09 (1)
22-21	-243 / 217	-17.5	-17.5	0.06(8)	6.25	2-21	-991 / 2	400	0.53 (1)
21-20	-1209 / 2393	-17.5	-17.5	0.41 (1)	5.82	13-11	-887 / 2	381	0.53 (1)
20-19	-1151 / 2492	-17.5	-17.5	0.43 (1)	5.93				OEIV/ED
19-18	-1509 / 3426	-17.5	-17.5	0.59(1)	5.35			R	ECEIVED
18-17	-1509 / 3426	-17.5	-17.5	0.59 (1)	5.35		I	OWI	N OF MILTON
17-16	-1044 / 2585	-17.5	-17.5	0.47 (1)	6.15				
16-15	-742 / 2234	-17.5	-17.5	0.42(1)	6.25			MΑ	R 29, 2017
15-14	-742 / 2234	-17.5	-17.5	0.42(1)	6.25				•
14-13	-850 / 2333	-17.5	-17.5	0.42 (1)	6.25				17-4978
13-12	-11 / 23	-17.5	-17.5	0.12 (11	6.25				
							В	UILD	ING DIVISION
1									

# **DESIGN CRITERIA**

SPECIFIED LOADS: LL = DL = LL = DL = AD = CH. 3.0 PSF 7.0 PSF TOTAL LOAD 33.3

### SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

TOTAL WEIGHT = 165 lb

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09

- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.28") CALCULATED VERT. DEFL.(LL)= L/ 999 (0.22") ALLOWABLE DEFL.(TL)= L/360 (1.28") CALCULATED VERT. DEFL.(TL)= L/999 (0.39")

CSI: TC=0.46 (5-6:7) , BC=0.59 (17-19:1) , WB=0.92 (5-17:3) , SSI=0.18 (10-11:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

### NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (4) (INPUT = 0.90) JSI METAL= 0.88 (18) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB PESC GREENPARK-LECCO RIDGE-BLOCK 327	,	DRWG NO.
TW0317-048	T59	1	1	TR-GREENPARK-LECCO RIDGE-BLOCK 327 TRUSS DESC.		Page 130 of 159 TW0317-048
Kott Lumber Uxbridge, Stouffville	, ON, TW	11	•	Version 8.100 S Feb 9 2	 2017 MiTe	ek Industries, Inc. Fri Mar 10 14:21:01 2017 Page 2
				ID:4BORDhR74Ei?Dog_xdfUkJyKZiL	.5azi9ry	TzEjfSK6yg5Q8vE3zNjIJOE6LFpcN9zcJJW
	,	WIND LOAD AP	PLIED IS DERIV	ED FROM REFERENCE VELOCITY PRESSURE OF ( 9.0) PSF AT		
		COEFFICIENTS	SX REFERENCE S, CpCg, BASED	ED FROM REFERENCE VELOCITY PRESSURE OF ( 9.0) PSF AT HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL N DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY		
		(OPEN TERRAL FROM EAVE.	N), AND TRUSS	IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY		
		PROWLAVE.				



JOB PESC TR-GREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY PLY DRWG NO Page 131 of 159 TRUSS DESC TW0317-048 TW0317-048 T60 Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:21:02 2017 Page Kott Lumber Uxbridge, Stouffville, ON, TW ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-pH8MvVsaEGMaGcvIWOdfh6nEpn5l2xFFZvZ9vbzcJJV 1.38 9-11-5 4-7-0 1680 7-2-3 SCALE = 1:79.4 4x5 Ⅱ 4x4 < 3x4 // 5x8 = 5x6 / 3x6 < 8 6.00 12 3x4 > ਜ 3x4 / 10 W10 **T**5 6-1-10 4x8 W 4x8 = wiз W 5 W14 15 17 16 21 20 19 14 13 3x6 = 2x4 || 3x4 = **4**x6 = **5x6** = 3x4 = 4x6 = 2x4 || 5x6 = 3x6 = 37-7-8 38-4-8 TOTAL WEIGHT = 167 lb DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER **DESIGN CRITERIA** BEARINGS FACTORED MAXIMUM FACTORED INPLIT REQRD SPECIFIED LOADS: LL = DL = LL = DL = AD = GROSS REACTION GROSS REACTION BRG CH. PSF **BRG** HORZ UPLIFT IN-SX 3.0 .IT VFRT HOR7 DOWN IN-SX PSF 21 12 1925 260 5-8 5-8 HANGER BY OTHERS 1820 1856 -770 7.0 PSF MIN. SEAT SIZE: 3-8 TOTAL LOAD 33.3 PSF PROVIDE ANCHORAGE AT BEARING JOINT 21 FOR 898 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 12 FOR 770 LBS FACTORED UPLIFT SPACING = 24.0 IN. C/C ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES. LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12 SHALL BE PROVIDED BY BUILDG. DESIGNER PROVIDE FOR 260 LBS FACTORED HORIZONTAL REACTION AT JOINT 21 THIS TRUSS IS DESIGNED FOR RESIDENTIAL

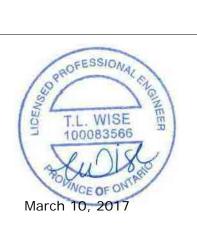
LUMBER				
N. L. G. A. R	ULES			
CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 5	2x4	DRY	No.2	SPF
5 - 7	2x4	DRY	No.2	SPF
7 - 9	2x4	DRY	No.2	SPF
9 - 11	2x4	DRY	No.2	SPF
21 - 2	2x4	DRY	No.2	SPF
12 - 11	2x4	DRY	No.2	SPF
21 - 17	2x4	DRY	No.2	SPF
17 - 15	2x4	DRY	No.2	SPF
15 - 12	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PL	PLATES (table is in inches)									
JT	TYPE	PLATES	W	LEN	Υ	X				
2	TMVW-t	MT20	4.0	8.0	1.75	3.00				
3	TMWW-t	MT20	3.0	4.0	1.50	1.75				
4	TTWW-m	MT20	5.0	8.0	2.25	3.75				
5	TTW-h	MT20	5.0	6.0	2.75	3.25				
6	TMWW-t	MT20	4.0	4.0	1.75	1.00				
7	TTW+p	MT20	4.0	5.0	2.25	2.00				
8	TMWW+t	MT20	3.0	4.0	1.75	0.75				
9	TS-t	MT20	3.0	6.0						
10	TMWW-t	MT20	3.0	4.0	1.50	1.75				
11	TMVW-t	MT20	4.0	8.0	1.75	Edge				
12	BMV1+p	MT20	2.0	4.0	2.25	1.00				
13	BMWW-t	MT20	4.0	6.0	1.75	1.75				
14	BMWW-t	MT20	3.0	4.0						
15	BS-t	MT20	3.0	6.0						
16	BMWWW-t	MT20	5.0	6.0	2.25	3.00				
17	BS-t	MT20	3.0	6.0						
18	BMWWW-t	MT20	5.0	6.0	2.00	1.50				
19	BMWW-t	MT20	3.0	4.0						
20	BMWW-t	MT20	4.0	6.0	1.75	1.75				
21	BMV1+p	MT20	2.0	4.0	2.25	1.00				

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.



UNFACTORED REACT	IONS	

	1ST LCASE	MAX./	MIN. COMPON	IENT REACTION	DNS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
21	1349	957 / 0	0/0	0/0	118 / -893	392 / 0	0/0		
12	1277	893 / 0	0/0	0/0	92 / -797	384 / 0	0/0		
HOR	HORIZONTAL REACTIONS								
21		0/0	0/0	0/0	186 / -152	0/0	0 /0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 21

<u>BRACING</u> TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.77 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 5.73 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 5-18. DBS = 8-0-0. CBF = 85 LBS. 1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 6-18, 8-16. DBS = 20-0-0 . CBF = 79 LBS.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 6-16. DBS = 12-0-0 . CBF = 81 LBS. 1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 7-16. DBS = 18-0-0 . CBF = 87 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S), USING (0.122"X3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING TOTAL LOAD CASES: (11)

	ORDS				WE		
	X. FACTORED	FACTO				MAX. FACTO	
MEMB.	FORCE	VERT. LO	AD LC1 MAX	MAX.	MEMB.	FORCE	MAX
1	(LBS)		.F) CSI (LC)			(LBS)	CSI (LC)
FR-TO		FROM	TO	LENGTH	H FR-TO		
1- 2	0 / 23	-77.3	-77.3 0.10 (1	) 10.00	20-3	-332 / 252	0.07 (1)
	-2770 / 1260		-77.3 0.45 (7			-177 / 245	0.11 (3)
3- 4	-2660 / 1266	-77.3	-77.3 0.45 (7	4.03	19- 4	-99 / 185	0.06 (7)
4- 5	-2875 / 1359	-77.3	-77.3 0.38 (7	3.89	4-18	-352 / 852	0.35 (8)
5- 6	-3234 / 1589					1746 / 936	0.42 (1)
6- 7	-2097 / 1053	-77.3	-77.3 0.29 (7	) 4.58	18- 6	-709 / 1311	0.33 (7)
7-8	-2105 / 1053	-77.3	-77.3 0.41 (8	) 4.42	6-16 -	1133 / 753	0.53 (3)
8- 9	-2495 / 1099	-77.3	-77.3 0.44 (8	) 4.11	16- 7	-777 / 1584	0.47 (7)
9-10	-2495 / 1099	-77.3	-77.3 0.44 (8	) 4.11	16-8	-618 / 510	0.30 (4)
10-11	-2614 / 1087	-77.3	-77.3 0.44 (8	3.97	14-8	-80 / 180	0.07 (8)
21- 2	-1931 / 920	0.0	0.0 0.19 (1	) 6.11	14-10	-166 / 243	0.14 (4)
12-11	-1813 / 796	0.0	0.0 0.18 (1	6.26	13-10	-340 / 250	0.09 (1)
						1021 / 2520	
21-20	-243 / 217	-17.5	-17.5 0.10 (1	1) 6.25	13-11	-8 7 / 2382	EČĚÍVED
20-19	-1250 / 2538	-17.5	-17.5 0.43 (1	5.73		K	ECEIVED
19-18	-1050 / 2395	-17.5	-17.5 0.41 (1	) 6.11		TOW	N OF MILTON
18-17	-919 / 2362	-17.5	-17.5 0.48 (1	) 6.25			
17-16	-919 / 2362	-17.5	-17.5 0.48 (1	) 6.25		M	AR 29, 2017
16-15	-743 / 2233	-17.5	-17.5 0.46 (1	) 6.25			•
15-14	-743 / 2233	-17.5	-17.5 0.46 (1	6.25			17-4978
14-13	-850 / 2334	-17.5	-17.5 0.42 (1	6.25			
13-12	-11 / 23	-17.5	-17.5 0.13 (1	1) 6.25		BUILI	DING DIVISION

OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09

- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.28") CALCULATED VERT. DEFL.(LL)= L/ 999 (0.22") ALLOWABLE DEFL.(TL)= L/360 (1.28") CALCULATED VERT. DEFL.(TL)= L/999 (0.41")

CSI: TC=0.45 (3-4:7), BC=0.48 (16-18:1), WB=0.55 (2-20:1) , SSI=0.18 (10-11:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

### NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

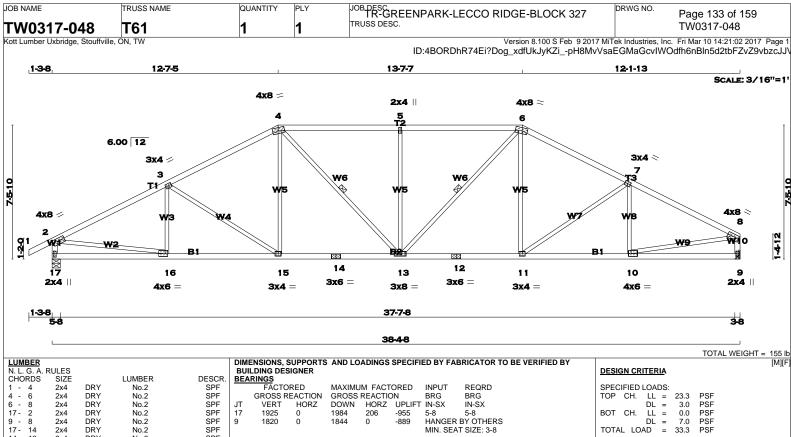
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (2) (INPUT = 0.90) JSI METAL= 0.66 (17) (INPUT = 1.00)

OB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESCRIPTION DIVISION DIDOE DI COMPANI	DRWG NO.
TW0317-048	T60	1	1	JOB DESC.  TR-GREENPARK-LECCO RIDGE-BLOCK 327 TRUSS DESC.	Page 132 of 159 TW0317-048
Kott Lumber Uxbridge, Stouffville	e, ON, TW		•	Version 8.100 S Feb 9 2017	MiTek Industries, Inc. Fri Mar 10 14:21:02 2017 Page 2
				ID:4BORDhR74Ei?Dog_xdfUkJyKZipH8Mv\ 	/saEGMaGcvIWOdfh6nEpn5l2xFFZvZ9vbzcJJV
		WIND LOAD AF	PLIED IS DERI	VED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT	
		(40-0-0) FT-IN-	SX REFERENC S, CpCg, BASE	VED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT EBHORT ABOVE GRADE AND USING EXTERNAL PEAK D ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON SIS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY	
		(OPEN TERRA FROM EAVE.	IN}, AND TRUS	ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON SS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY	
		FROM EAVE.			





LUMBER				
N. L. G. A. R	ULES			
CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 6	2x4	DRY	No.2	SPF
6 - 8	2x4	DRY	No.2	SPF
17 - 2	2x4	DRY	No.2	SPF
9 - 8	2x4	DRY	No.2	SPF
17 - 14	2x4	DRY	No.2	SPF
14 - 12	2x4	DRY	No.2	SPF
12- 9	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

l	PLATES (table is in inches)									
l	JT	TYPE	PLATES	W	LEN	Υ	Χ			
l	2	TMVW-t	MT20	4.0	8.0	1.75	3.00			
l	3	TMWW-t	MT20	3.0	4.0	1.50	1.75			
l	4	TTWW-m	MT20	4.0	8.0	1.75	3.75			
l	5	TMW+w	MT20	2.0	4.0					
l	6	TTWW-m	MT20	4.0	8.0	1.75	3.50			
l	7	TMWW-t	MT20	3.0	4.0	1.50	1.75			
l	8	TMVW-t	MT20	4.0	8.0	1.75	Edge			
l	9	BMV1+p	MT20	2.0	4.0	2.25	1.00			
l	10	BMWW-t	MT20	4.0	6.0	1.75	1.75			
l	11	BMWW-t	MT20	3.0	4.0					
l	12	BS-t	MT20	3.0	6.0					
l	13	BMWWW-t	MT20	3.0	8.0					
l	14	BS-t	MT20	3.0	6.0					
l	15	BMWW-t	MT20	3.0	4.0					
l	16	BMWW-t	MT20	4.0	6.0	1.75	1.75			
I	17	BMV1+p	MT20	2.0	4.0	2.25	1.00			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

T.L. WISE TO 100083566

100083566

NCEOFON

March 10, 2017

	BEARINGS								
	FACTOR	RED	MAXIMUM FACTORED			INPUT	REQRD		
	GROSS RE	ACTION	GROSS REACTION			BRG	BRG		
JΤ	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX		
17	1925	0	1984	206	-955	5-8	5-8		
9	1820	0	1844	0	-889	HANGER E	BY OTHERS		
						MIN. SEAT	SIZE: 3-8		

PROVIDE ANCHORAGE AT BEARING JOINT 17 FOR 955 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 9 FOR 889 LBS FACTORED UPLIFT UPLIFT TOTAL LOAD

PART 9, NBCC 2010

SPACING =

- TPIC 2011

ROOF LIVE LOAD

33.3 PSF

IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF

- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED

ALLOWABLE DEFL.(LL)= L/360 (1.28") CALCULATED VERT. DEFL.(LL)= L/999 (0.17") ALLOWABLE DEFL.(TL)= L/360 (1.28") CALCULATED VERT. DEFL.(TL)= L/999 (0.31")

CSI: TC=0.65 (2-3:7), BC=0.48 (15-16:1), WB=0.79 (2-16:7), SSI=0.26 (4-5:1)

COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg.

TRUSS MANUFACTURING PLANT

JSI GRIP= 0.89 (8) (INPUT = 0.90) JSI METAL= 0.64 (14) (INPUT = 1.00)

NAIL VALUES

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

24.0

THIS DESIGN COMPLIES WITH:

ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES. SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 206 LBS FACTORED HORIZONTAL REACTION AT JOINT 17

UNFACTORED REAC	CTIONS		
19T   CASE	MAY /MINI	COMPONENT	PEACTIONS

	IOI LUAGE	MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
17	1349	957 / 0	0/0	0/0	148 / -934	392 / 0	0/0	
9	1277	893 / 0	0/0	0/0	61 / -882	384 / 0	0/0	
HOR 17	IZONTAL REA	ACTIONS 0/0	0/0	0/0	147 / -121	0/0	0 /0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 17

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.69 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 5.65 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 4-13, 6-13. DBS = 20-0-0 . CBF = 45 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

WIN

LOADING TOTAL LOAD CASES: (11)

ı	767722678 67626.(17)									
	СН	ORDS					WE	BS		
ı	MAX	. FACTORED	FACTOR	RED				MAX. FACTO	ORED	
ı	MEMB.	FORCE	VERT. LOA	AD LC1	I MAX	MAX.	MEMB.	FORCE	MAX	
ı		(LBS)	(PLF	F) (	CSI (LC)	UNBRAC		(LBS)	CSI (LC)	
ı	FR-TO	, ,	FROM 7			LENGTH		, ,	, ,	
ı	1- 2	0 / 23	-77.3	-77.3	0.10(1)	10.00	16-3	-228 / 243	0.06 (1)	
	2-3	-2856 / 1388	-77.3	-77.3	0.65 (7)	3.69	3-15	-433 / 397	0.48 (3)	
	3- 4	-2520 / 1310	-77.3	-77.3	0.64 (7)	3.94	15- 4	-141 / 351	0.14 (7)	
	4- 5	-2582 / 1385	-77.3	-77.3	0.61 (7)	3.77	4-13	-383 / 509	0.26 (8)	
	5- 6	-2582 / 1385	-77.3	-77.3	0.61 (7)	3.77	13- 5	-682 / 527	0.67 (3)	
	6- 7	-2448 / 1284	-77.3	-77.3	0.60 (8)	4.02	13-6	-411 / 602	0.28 (7)	
	7-8	-2648 / 1305	-77.3	-77.3	0.60 (8)	3.85	11-6	-115 / 299	0.11 (8)	
	17- 2	-1935 / 985	0.0	0.0	0.19 (1)	6.11	11- 7	-325 / 331	0.34 (4)	
	9- 8	-1797 / 918	0.0	0.0	0.18 (1)	6.27		-309 / 278	0.09 (1)	
								-1114 / 2590	0.79 (7)	
		-188 / 160			0.16 (11		10-8	-1080 / 2429	0.71 (8)	
	16-15	-1288 / 2608			0.48 (1)					
		-956 / 2249	-17.5	-17.5	0.43 (1)	6.25				
	14-13	95 / 2:				6.25				
	13-12	- /2 /2	17 17	- 5	0.4 1) 0.4 1)	6.25				
	12-11	- /2	12	- 5		6.25				i
	11-10	= 104 / 2: 3	1/	- 5	0.4 1)	6.12		D	ECEIVED	
	10-9	11/25		5	0.1 11	) 6.25				
	_							IOW	N OF MILTON	
	MANAGE F	READ ALL NO	TES ON TH	HIS PA	GE ANI	O ON TH	ΙE	DECCUPE	MT (200) OCT TT	
		NGINEERING						KESSUKILI	AGR (299) 261 AT	

EXTERNAL PEAK ING SYSTEM): M9778NAL

MAY BE LOCATED ON EABITHLED HINGS BAY WASHON

ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE

IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED

IN THE DESIGN OF THIS COMPONENT.

POBPES GREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY PLY DRWG NO. Page 134 of 159 TW0317-048 TW0317-048 T62 Kott Lumber Uxbridge, Stouffville, ON, TW Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:21:03 2017 Page 1 ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-HThk6rtC?aURulUV458uEKKIVBQDnOHOoZliR2zcJJU 15-3-5 149-13 1-3-8

SCALE = 1  6.00 12  3x4 = 4x4   7  3x6   7  3x4 = 4x4   7  3x6   7  3x4   4x8   7  3x4   4x8   7  3x4   4x8   7  3x8   7  3x4   4x8   7  3x8   7  3	1.00	1000		<i>F1</i>	140-10	
3x4 4 4 8 3x4 9 9 10 N			4x4 🤛			SCALE =
3x6		6.00 1	<del>2</del>	<b>4x4</b> ≥		
3x6						
3		. /	// 11 //	// //		
10 10 10 10 10 10 10 10 10 10 10 10 10 1	т	3	WS	Wa Wa	<b>T4</b>	
16	2	wз			ws	
16		<b>B</b> J		2	B(3	4
1-3-8 5-8 38-4-8	<u></u> 16	15	14	13	12	
1-3-8 1 5-8 38-4-8	2x4					2x4
38-4-8						
38-4-8	1-3-8		37:	7-8		
38-4-8	5-8					3-8
			39.4	LO		
				ro		TOTAL WEIGHT

LUMBER					
N. L. G. A. R	RULES				
CHORDS	SIZE		LUMBER	DESCR.	
1 - 4	2x4	DRY	No.2	SPF	
4 - 5	2x4	DRY	No.2	SPF	
5 - 7	2x4	DRY	No.2	SPF	
7 - 8	2x4	DRY	No.2	SPF	
8 - 10	2x4	DRY	No.2	SPF	
16 - 2	2x4	DRY	No.2	SPF	
11 - 10	2x4	DRY	No.2	SPF	
16 - 14	2x4	DRY	No.2	SPF	
14 - 13	2x4	DRY	No.2	SPF	
13 - 11	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

PL	PLATES (table is in inches)									
JT	TYPE	PLATES	W	LEN	Υ	Χ				
2	TMVW-t	MT20	4.0	8.0	1.75	3.00				
3	TMWW-t	MT20	3.0	4.0	1.50	1.75				
4	TS-t	MT20	3.0	6.0						
5	TTW-h	MT20	4.0	4.0	2.00	1.75				
6	TMWW-t	MT20	3.0	4.0						
7	TTW-h	MT20	4.0	4.0	2.00	1.75				
8	TS-t	MT20	3.0	6.0						
9	TMWW-t	MT20	3.0	4.0	1.50	1.75				
10	TMVW-t	MT20	4.0	8.0	1.75	Edge				
11	BMV1+p	MT20	2.0	4.0	2.25	1.00				
12	BMWW-t	MT20	4.0	5.0	1.50	1.50				
13	BSWWW-I	MT20	5.0	8.0	3.00	4.00				
14	BSWWW-I	MT20	5.0	8.0	3.00	4.00				
15	BMWW-t	MT20	4.0	6.0	1.75	2.00				
16	BMV1+p	MT20	2.0	4.0	2.25	1.00				

Edge - INDICATES REFERENCE CORNER OF PLATE

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS	AND LOADINGS S	SPECIFIED BY FAB	RICATOR TO B	E VERIFIED BY
BUILDING DESIGNER				
BEARINGS				

DEA	DEARINGS								
	FACTOR	ED	MAXIMUM FACTORED			INPUT	REQRD		
	<b>GROSS RE</b>	GROSS REACTION			BRG	BRG			
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX		
16	1925	0	1977	236	-924	5-8	5-8		
11	1820	0	1852	0	-856	HANGER E	BY OTHERS		
						MIN. SEAT	SIZE: 3-8		

PROVIDE ANCHORAGE AT BEARING JOINT 16 FOR 924 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 11 FOR 856 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 236 LBS FACTORED HORIZONTAL REACTION AT JOINT 16

### UNFACTORED REACTIONS

	1ST LCASE	MAX./	MIN. COMPON	IENT REACTION	DNS		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
16	1349	957 / 0	0/0	0/0	132 / -912	392 / 0	0/0
11	1277	893 / 0	0/0	0/0	79 / -858	384 / 0	0/0
HOR	HORIZONTAL REACTIONS						
16		0/0	0/0	0/0	169 / -139	0/0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 16

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.28 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 5.68 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

- 1 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 3-14, 5-14, 6-14, 6-13, 7-13, 9-13. DBS = 20-0-0 . CBF = 79 LBS.
- 1 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 2-15. DBS = 10-0-0 . CBF = 79 LBS. 1 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 10-12. DBS = 12-0-0 . CBF = 91 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING TOTAL LOAD CASES: (11)

	СН	ORDS					WE	ВЅ			
ı	MAX	. FACTORED	FACTOR	ED				MAX	(. FACT	ORED	
ı	MEMB.	FORCE	VERT. LOA	AD LC1	MAX	MAX.	MEMB		FORCE	MAX	
ı		(LBS)	(PLF	F) C	SI (LC)	<b>UNBRAC</b>			(LBS)	CSI (LC)	
	FR-TO		FROM 1	ľΟ		LENGTH	FR-TO				
	1- 2	0 / 23	-77.3	-77.3	0.10(1)	10.00	15-3	-141	/ 227	0.05(3)	
	2-3	-2859 / 1338	-77.3	-77.3	0.90(1)	3.28	3-14	-674	1/558	0.37 (3)	
	3- 4	-2306 / 1166	-77.3	-77.3	0.84 (7)	3.72	14- 5	-285	653	0.14(1)	
	4- 5	-2306 / 1166	-77.3	-77.3	0.84 (7)	3.72	14- 6	-208	3 / 277	0.13 (4)	
	5- 6	-2043 / 1143	-77.3	-77.3	0.31 (7)	4.63	6-13	-273	3 / 298	0.17(3)	
	6- 7	-2001 / 1127			0.31 (8)		13- 7	-288	3 / 658	0.14(1)	
	7-8	-2266 / 1152			(8) 08.0		13- 9	-569	9 / 498	0.30 (4)	
	8- 9	-2266 / 1152			(8) 08.0				2 / 254	0.08 (1)	
	9-10	-2683 / 1270			0.81 (8)		2-15	-1048	3 / 2590	0.57 (1)	
	16- 2	-1920 / 961	0.0		0.19 (1)	6.13	12-10	-1026	6 / 2451	0.54 (1)	
	11-10	-1796 / 893	0.0	0.0	0.18 (1)	6.28					
	16-15	-219 / 193			0.23 (11)						
	15-14	-1261 / 2622			0.52 (1)						
	14-13	-811 / 2131			0.45 (1)				D	CCEN/	_
	13-12	-994 / 2426			0.50 (1)					ECEIVE	
	12-11	-11 / 23	-17.5	-17.5	0.22 (11)	6.25			TOW	/N OF MI	Ľ
										AD 20 20	

D TON MAR 29, 2017 17-4978 **BUILDING DIVISION** 

### **DESIGN CRITERIA**

SPECIFIED LOADS: PSF

### SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09

- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.28")
CALCULATED VERT. DEFL.(LL) = L/999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (1.28")
CALCULATED VERT. DEFL.(TL) = L/999 (0.33")

CSI: TC=0.90 (2-3:1), BC=0.52 (14-15:1), WB=0.57 (2-15:1) , SSI=0.26 (2-3:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

### NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (5) (INPUT = 0.90) JSI METAL= 0.73 (12) (INPUT = 1.00)



OB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC TR-GREENPARK-LECCO RIDGE-BLOCK 327	DRWG NO. Page 135 of 159
ΓW0317-048	T62	1	1	THOOG BEGG.	TW0317-048
ott Lumber Uxbridge, Stouffville	e, ON, TW			Version 8.100 S Feb. 9 2017 ID:4BORDhR74Ei?Dog_xdfUkJyKZiHThk6rt	7 MiTek Industries, Inc. Fri Mar 10 14:21:03 2017 Page 2 :C?aURuIUV458uEKKIVBQDnOHOoZIiR2zcJJU
		WIND LOAD API	PLIED IS DERIV	ED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL N DESIGN (CATEGORY 2), BUILDING MAY BE LOCATED ON IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY	
		WIND PRESSU	RE IS BASED O	ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL N DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON	
		FROM EAVE.	N}, AND TRUSS	TIS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY	

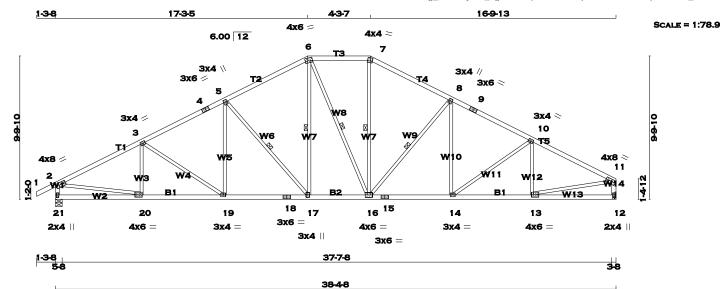


JOB PR-GREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY TRUSS DESC. TW0317-048 T63 Kott Lumber Uxbridge, Stouffville, ON, TW

DRWG NO.

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LUMBER				
N. L. G. A. R	ULES			
CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 6	2x4	DRY	No.2	SPF
6 - 7	2x4	DRY	No.2	SPF
7 - 9	2x4	DRY	No.2	SPF
9 - 11	2x4	DRY	No.2	SPF
21 - 2	2x4	DRY	No.2	SPF
12 - 11	2x4	DRY	No.2	SPF
21 - 18	2x4	DRY	No.2	SPF
18 - 15	2x4	DRY	No.2	SPF
15 - 12	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER

PL	PLATES (table is in inches)							
JT	TYPE	PLATES	W	LEN	Υ	X		
2	TMVW-t	MT20	4.0	8.0	1.75	3.00		
3	TMWW-t	MT20	3.0	4.0	1.50	1.75		
4	TS-t	MT20	3.0	6.0				
5	TMWW+t	MT20	3.0	4.0	1.75	0.75		
6	TTWW-m	MT20	4.0	6.0	1.75	2.25		
7	TTW-m	MT20	4.0	4.0	2.00	1.75		
8	TMWW+t	MT20	3.0	4.0	1.75	0.75		
9	TS-t	MT20	3.0	6.0				
10	TMWW-t	MT20	3.0	4.0	1.50	1.75		
11	TMVW-t	MT20	4.0	8.0	1.75	Edge		
12	BMV1+p	MT20	2.0	4.0	2.25	1.00		
13	BMWW-t	MT20	4.0	6.0	1.75	1.75		
14	BMWW-t	MT20	3.0	4.0				
15	BS-t	MT20	3.0	6.0				
16	BMWWW-t	MT20	4.0	6.0				
17	BMWW+t	MT20	3.0	4.0				
18	BS-t	MT20	3.0	6.0				
19	BMWW-t	MT20	3.0	4.0				
20	BMWW-t	MT20	4.0	6.0	1.75	1.75		
21	BMV1+p	MT20	2.0	4.0	2.25	1.00		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.



DIMENSIONS, SUPPORTS	AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY
BUILDING DESIGNER	
DEVDINGS	

DLA	SEARINGS											
	FACTO	RED	MAXIMU	IM FACT	ORED	INPUT	REQRD					
	GROSS R	EACTION	GROSS	REACTIO	N	BRG	BRG					
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX					
21	1925	0	1972	262	-896	5-8	5-8					
12	1820	0	1857	0	-828	HANGER I	BY OTHERS					
						MIN. SEAT	SIZE: 3-8					

PROVIDE ANCHORAGE AT BEARING JOINT 21 FOR 896 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 12 FOR 828 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES. SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 262 LBS FACTORED HORIZONTAL REACTION AT JOINT 21

UNFACTORED REACT	TIONS
10710105	

	1ST LCASE	MAX./	MIN. COMPO	NENT REACTION	ONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL			
21	1349	957 / 0	0/0	0/0	117 / -892	392 / 0	0/0			
12	1277	893 / 0	0/0	0/0	92 / -838	384 / 0	0/0			
HOR	HORIZONTAL REACTIONS									
21		0/0	0/0	0/0	187 / -153	0/0	0 /0			

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 21

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.82 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 5.74 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 5-17, 6-17, 6-16, 7-16, 8-16. DBS = 20-0-0 . CBF = 84 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING TOTAL LOAD CASES: (11)

IOIAL	TOTAL EURD GROES. (11)									
СН	CHORDS WEBS									
MAX	(. FACTORED	FACTO	RED				MAX. FAC	TORED		
MEMB.	FORCE	VERT. LO	AD LC	1 MAX	MAX.	MEMB.	FORCE	E MAX		
	(LBS)	(PL	.F) (	CSI (LC)	<b>UNBRAG</b>	2	(LBS)	CSI (LC)		
FR-TO		FROM	TO		LENGTH	FR-TO				
1-2	0 / 23	-77.3	-77.3	0.10(1)	10.00	20-3	-260 / 231	0.06 (1)		
2-3	-2803 / 1264	-77.3	-77.3	0.48 (7)	3.82	3-19	-268 / 284	0.23 (3)		
3-4	-2586 / 1241	-77.3	-77.3	0.48 (7)	4.05	19- 5	-88 / 253	0.07 (7)		
4- 5	-2586 / 1241	-77.3	-77.3	0.48 (7)	4.05	5-17	-711 / 541	0.37 (3)		
5-6	-2098 / 1080	-77.3	-77.3	0.45 (7)		17- 6	-344 / 618	0.22 (7)		
6- 7	-1849 / 1038			0.32 (8)		6-16	-132 / 150	0.10 (8)		
7-8	-2082 / 1073			0.43 (8)		16- 7	-255 / 581	0.16 (7)		
8- 9	-2492 / 1206	-77.3	-77.3	0.46(8)	4.11	16-8	-643 / 505	0.33 (4)		
9-10	-2492 / 1206	-77.3	-77.3	0.46(8)	4.11	14-8	-66 / 197	0.06 (8)		
10-11	-2616 / 1183	-77.3	-77.3	0.46(8)	3.96	14-10	-171 / 233	0.14 (4)		
21- 2	-1926 / 923	0.0	0.0	0.19(1)	6.12	13-10	-338 / 264	0.09 (1)		
12-11	-1813 / 855	0.0	0.0	0.18 (1)	6.26	2-20 -	1006 / 2540	0.58 (7)		
						13-11	-973 / 2384	0.53 (8)		
21-20	-245 / 219	-17.5	-17.5	0.13 (11	) 6.25					
20-19	-1241 / 2567			0.45 (1)						
19-18	-1005 / 2344	-17.5	-17.5	0.42(1)	6.23					
18-17	-1005 / 2344	-17.5	-17.5	0.42(1)	6.23					
17-16	-654 / 1884	-17.5	-17.5	0.35 (1)	6.25		_ r	RECEIVED		
16-15	-745 / 2230	-17.5	-17.5	0.41 (1)	6.25		TO	WN OF MILTON		
15-14	-745 / 2230	-17.5	-17.5	0.41 (1)	6.25					
14-13	-935 / 2335	-17.5	-17.5	0.42 (1)	6.25			//AR 29, 2017		
13-12	-11 / 23	-17.5	-17.5	0.12 (11	) 6.25			· · · · · · · · · · · · · · · · · · ·		
								17-4978		
							BUII	DING DIVISION		
							DOI	-DITTO DIVIDIDIT		

### **DESIGN CRITERIA**

SPECIFIED LOADS: LL = DL = LL = DL = AD = CH. 3.0 PSF 7.0 PSF TOTAL LOAD 33.3

### SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

TOTAL WEIGHT = 169 lb

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09

- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.28") CALCULATED VERT. DEFL.(LL)= L/ 999 (0.15") ALLOWABLE DEFL.(TL)= L/360 (1.28") CALCULATED VERT. DEFL.(TL)= L/999 (0.28")

CSI: TC=0.48 (2-3:7), BC=0.45 (19-20:1), WB=0.58 (2-20:7), SSI=0.19 (2-3:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

### NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (7) (INPUT = 0.90) JSI METAL= 0.63 (18) (INPUT = 1.00)

ов NAME Г <b>W0317-048</b>	TRUSS NAME	QUANTITY	PLY 1	JOB DESC. TRUSS DESC.	OCK 327	DRWG NO. Page 137 of 159 TW0317-048
ott Lumber Uxbridge, Stouffville	e, ON, TW	I		Version 8.11	00 S Feb 9 2017 Mi	Tek Industries, Inc. Fri Mar 10 14:21:04 2017 Page 2
ott Lumber Uxbridge, Stouffville	e, ON, TW			Version 8.11  ID:4BORDhR74Ei?Dog_xdfUkJyh  /ED FROM REFERENCE VELOCITY PRESSURE OF { 9.0}  /ED HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK  ON THE (MAIN WIND FORCE RESISTING SYSTEM).INT  IN DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED  S IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-S	KZilgF6KBuqm	Tek Industries, Inc. Fri Mar 10 14:21:04 2017 Page 2 uclWv2hepf7mXsanbnZWqPY1D2G_UzcJJT

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

JOB NAME TRUSS NAME T64

QUANTITY PLY JOB PESC TR-GREENPARK-LECCO RIDGE-BLOCK 327 TRUSS DESC

DRWG NO

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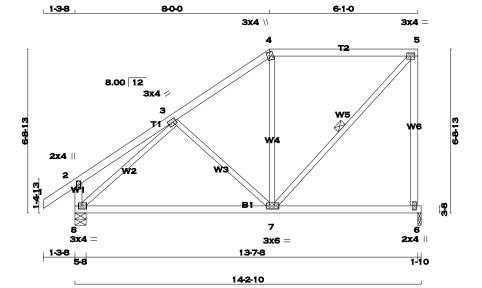
SCALE = 1:47.3

TOTAL WEIGHT = 64 lb

Kott Lumber Uxbridge, Stouffville, ON, TW

TW0317-048

Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:21:04 2017 Page 1 ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-lgF6KBuqmuclWv2hepf7mXsYrbqzWusY1D2G\_UzcJJ7



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR 4 2x4 DRY No.2 No.2 SPF SPF 2x4 DRY 6 8 5 2x4 DRY No 2 SPF No.2 8 6 2x4 DRY No.2 SPF ALL WEBS DRY No.2 SPF 2x3 EXCEPT

DRY: SEASONED LUMBER

PLATES (table is in inches)
-----------------------------

J١	TYPE	PLATES	VV	LEN	Υ	Х	
2	TMV+p	MT20	2.0	4.0			
3	TMWW-t	MT20	3.0	4.0	1.50	1.75	
4	TTW+m	MT20	3.0	4.0	2.00	1.25	
5	TMVW-t	MT20	3.0	4.0			
6	BMV1+p	MT20	2.0	4.0			
7	BMWWW-t	MT20	3.0	6.0	1.50	1.50	
8	BMVW1-t	MT20	3.0	4.0	1.50	1.75	

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY **BUILDING DESIGNER** 

BEA	RINGS						
	FACTORED		MAXIMUM FACTORED			INPUT	REQRD
	GROSS RI	EACTION	GROSS I	REACTIO	BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
6	668	0	726	0	-397	1-10	1-10
8	774	0	830	384	-372	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 6 FOR 397 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 8 FOR 372 LBS FACTORED UPLIFT

PROVIDE FOR 384 LBS FACTORED HORIZONTAL REACTION AT JOINT 8

### UNFACTORED REACTIONS

	1ST LCASE	MAX./I	MIN. COMPON	<u>NENT REACTIO</u>	ONS		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
6	469	328 / 0	0/0	0/0	145 / -374	141 / 0	0/0
8	541	392 / 0	0/0	0/0	139 / -361	149 / 0	0/0
HOR 8	IZONTAL RE	ACTIONS 0/0	0/0	0/0	274 / -181	0/0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 6, 8

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 5-7. DBS = 20-0-0 . CBF = 34 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING TOTAL LOAD CASES: (11)

СНО	DRDS				WEBS				
MAX.	FACTORED	FACTO	RED				MAX. FACTO	ORED	
MEMB.	FORCE	VERT. LO	AD LC	1 MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PL	.F) (	CSI (LC)	<b>UNBRAC</b>		(LBS)	CSI (LC)	
FR-TO		FROM	TO		LENGTH	FR-TO	, ,	, ,	
1- 2	0 / 29	-77.3	-77.3	0.10(1)	10.00	3- 7	-267 / 357	0.14 (3)	
2-3	-26 / 167	-77.3	-77.3	0.24 (7)	6.25	7- 4	-111 / 128	0.08(3)	
3- 4	-510 / 326	-77.3	-77.3	0.26 (7)	6.25	7- 5	-315 / 607	0.15 (7)	
4- 5	-415 / 340	-77.3	-77.3	0.39 (7)	6.25	8-3	-756 / 236	0.36 (4)	
6- 5	-692 / 420	0.0	0.0	0.61 (1)	7.81				
8- 2	-260 / 250	0.0	0.0	0.04 (7)	7.81				
8- 7	-378 / 610	-17.5	-17.5	0.30 (11	) 6.25				
7-6	-49 / 126	-17.5	-17.5	0.27 (11	) 6.25				

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF ( 9.0) PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCq, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

RECEIVED TOWN OF MILTON MAR 29, 2017 17-4978 **BUILDING DIVISION** 

# **DESIGN CRITERIA**

TOTAL LOAD

SPECIFIED LOADS: LL = DL = LL = DL = AD = PSF CH. 23.3 3.0 PSF 7.0 PSF

### SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

33.3 PSF

THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09

- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.47\*) CALCULATED VERT. DEFL.(LL)= L/ 999 (0.01\*) ALLOWABLE DEFL.(TL)= L/360 (0.47\*) CALCULATED VERT. DEFL.(TL)= L/999 (0.11\*)

CSI: TC=0.61 (5-6:1), BC=0.30 (7-8:11), WB=0.36 (3-8:4), SSI=0.18 (4-5:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

### NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (7) (INPUT = 0.90) JSI METAL= 0.26 (3) (INPUT = 1.00)



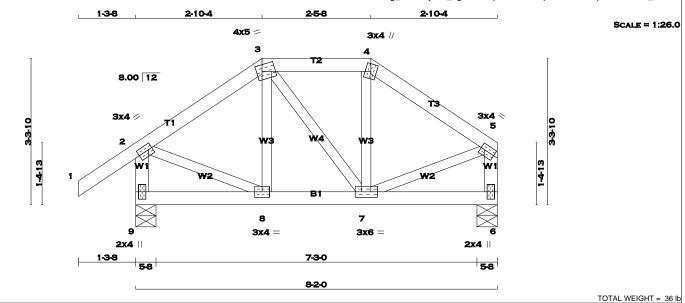
JOB NAME TRUSS NAME QUANTITY PLY TRUSS DESC. TW0317-048 T65 Kott Lumber Uxbridge, Stouffville, ON, TW

JOB PESC TR-GREENPARK-LECCO RIDGE-BLOCK 327

DRWG NO

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LUMBER N. L. G. A. CHORDS LUMBER DESCR SIZE 3 2x4 2x4 DRY No.2 No.2 SPF SPF DRY 5 2 2x4 DRY No 2 SPF 2x4 No.2 6 5 2x4 DRY No.2 SPF 6 DRY No.2 ALL WERS 2x3 DRY No.2 SPF EXCEPT

DRY: SEASONED LUMBER

PLATES (table is in inches)
JT TYPE PLATES LEN Y 4.0 5.0 4.0 4.0 4.0 1.50 1.00 1.75 1.50 TMVW-t MT20 TTWW-m TTW+m MT20 3.0 TMVW-t MT20 3.0 1.50 1.00 BMV1+p MT20 3.0 RMWWW-t MT20 6.0 BMWW-t 2.0 BMV1+p MT20 4.0

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TI 20 PLATES IS ALLOWED.

### DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEA	RINGS						
	FACTOR	ED	MAXIMU	M FACTO	INPUT	REQRD	
	<b>GROSS RE</b>	ACTION	GROSS REACTION			BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	<b>UPLIFT</b>	IN-SX	IN-SX
9	493	0	521	0	-237	5-8	5-8
6	387	0	403	161	-182	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 9 FOR 237 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 6 FOR 182 LBS FACTORED UPLIFT

PROVIDE FOR 161 LBS FACTORED HORIZONTAL REACTION AT JOINT 6

### UNFACTORED REACTIONS

	1ST LCASE	MAX./	MIN. COMPON	NENT REACTION	DNS		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
9	344	254 / 0	0/0	0/0	68 / -227	90 / 0	0/0
6	272	190 / 0	0/0	0/0	39 / -182	82 / 0	0/0
HOR 6	IZONTAL REA	ACTIONS 0/0	0/0	0/0	115 / -107	0/0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 9, 6

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

<u>LOADING</u> TOTAL LOAD CASES: (11)

l	CHC	RDS					WE	BS		
l	MAX.	FACTORED	FACTOR	RED				MAX. FACTO	RED	
l	MEMB.	FORCE	VERT. LO	AD LC	1 MAX	MAX.	MEMB.	FORCE	MAX	
l		(LBS)	(PL	F) (	CSI (LC)	<b>UNBRAC</b>		(LBS)	CSI (LC)	
l	FR-TO		FROM	TO		LENGTH	FR-TO			
l	1- 2	0 / 29	-77.3	-77.3	0.10(1)	10.00	8-3	-46 / 55	0.01 (8)	
l	2-3	-313 / 171	-77.3	-77.3	0.14 (7)	6.25	3- 7	-55 / 58	0.01 (5)	
l	3- 4	-260 / 196	-77.3	-77.3	0.08 (8)	6.25	7- 4	-50 / 77	0.01 (8)	
l	4- 5	-314 / 171	-77.3	-77.3	0.13 (8)	6.25	2-8	-60 / 277	0.06(1)	
l	9- 2	-498 / 252	0.0	0.0	0.05(1)	7.81	7- 5	-85 / 278	0.06(1)	
l	6- 5	-380 / 196	0.0	0.0	0.04(1)	7.81				
l										
l	9-8	-10 / 21	-17.5	-17.5	0.03 (11)	10.00				
l	8- 7	-48 / 258	-17.5	-17.5	0.05 (1)	6.25				
١	7-6	-128 / 151	-17.5	-17.5	0.03 (6)	6.25				

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT { 40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

### **DESIGN CRITERIA**

SPECIFIED LOADS: LL = DL = LL = DL = AD = CH. PSF 3.0 0.0 7.0 PSF PSF PSF TOTAL LOAD 33.3 PSF

### SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09

- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.27\*) CALCULATED VERT. DEFL.(LL)= L/ 999 (0.00\*) ALLOWABLE DEFL.(TL)= L/360 (0.27\*) CALCULATED VERT. DEFL.(TL)= L/ 999 (0.01\*)

CSI: TC=0.14 (2-3:7), BC=0.05 (7-8:1), WB=0.06 (2-8:1), SSI=0.08 (4-5:8)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

### NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (2) (INPUT = 0.90) JSI METAL= 0.16 (2) (INPUT = 1.00)





READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

JOB NAME TRUSS NAME QUANTITY PLY 2 TW0317-048 T66

JOB PESC TR-GREENPARK-LECCO RIDGE-BLOCK 327 TRUSS DESC.

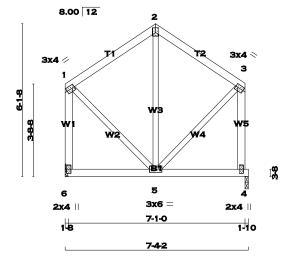
DRWG NO.

Page 140 of 159 TW0317-048

Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:21:05 2017 Page 1 Kott Lumber Uxbridge, Stouffville, ON, TW ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-DspUXXuSXBl973dtBWAMJIPp0\_DtFPUhFtnpWwzcJJ\$

> 3.7.8 3x4 ||

SCALE = 1:46.2



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR 2 2x4 2x4 DRY No.2 No.2 SPF SPF DRY 2x4 DRY No 2 SPF 6 4 6 2x4 No.2 2x4 DRY No.2 SPF ALL WEBS DRY No.2 SPF 2x3 EXCEPT

DRY: SEASONED LUMBER

PLATES (table is in inches)

JΤ	TYPE	PLATES	W	LEN	Υ	Х	
1	TMVW-t	MT20	3.0	4.0	1.50	1.00	
2	TTW+p	MT20	3.0	4.0	2.25	1.50	
3	TMVW-t	MT20	3.0	4.0	1.50	1.00	
4	BMV1+p	MT20	2.0	4.0			
5	BMWWW-t	MT20	3.0	6.0			
6	BMV1+p	MT20	2.0	4.0			

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

3EA	RINGS						
	FACTOR	RED	MAXIMUI	M FACTO	INPUT	REQRD	
	GROSS RE	ACTION	GROSS F	REACTIO	N	BRG	BRG
ΙT	VERT	HORZ	DOWN	HORZ	<b>UPLIFT</b>	IN-SX	IN-SX
3	342	0	380	290	-149	HANGER E	BY OTHERS
						MIN. SEAT	SIZE: 1-8
1	342	0	380	0	-150	1-10	1-10

PROVIDE ANCHORAGE AT BEARING JOINT 6 FOR 150 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 4 FOR 150 LBS FACTORED UPLIFT

PROVIDE FOR 290 LBS FACTORED HORIZONTAL REACTION AT JOINT 6

UNFACTORED REACTIONS

UIVI	ONI ACTORED REACTIONS											
	1ST LCASE	MAX./	MAX./MIN. COMPONENT REACTIONS									
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL					
6	240	168 / 0	0/0	0/0	94 / -153	72 / 0	0/0					
4	240	168 / 0	0/0	0/0	95 / -154	72 / 0	0/0					
HOF	RIZONTAL REA	ACTIONS 0/0	0/0	0/0	207 / -207	0/0	0 /0					

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 4

6

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (11)

CHC	RDS					W E	BS		
MAX.	FACTORED	FACTOR	RED				MAX. FACTO	DRED	
MEMB.	FORCE	VERT. LO.	AD LC	1 MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PL	F) (	CSI (LC)			(LBS)	CSI (LC)	
FR-TO		FROM	TO		LENGTH	FR-TO			
1-2	-161 / 157	-77.3	-77.3	0.20 (7)	6.25	5- 2	-131 / 25	0.08 (1)	
2-3	-161 / 158	-77.3	-77.3	0.19 (8)	6.25	1- 5	-108 / 185	0.04 (5)	
6- 1	-355 / 166	0.0	0.0	0.09 (8)	7.81	5-3	-109 / 187	0.04 (6)	
4-3	-355 / 167	0.0	0.0	0.09 (7)	7.81				
6- 5	-224 / 259	-17.5	-17.5	0.06 (11	6.25				
5- 4	-31 / 67	-17.5	-17.5	0.06 (11	6.25				

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF ( 9.0) PSE AT WIND LOAD APPLIED IS DERIVED FROM REPERSING VELOCITY FRESSURE OF (3.9), 25 AT (40-0.0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.

### **DESIGN CRITERIA**

TPIC 2011

SPECIFIED LOADS:

LL = DL = LL = DL = AD = PSF CH. 3.0 PSF PSF 7.0 PSF TOTAL LOAD 33.3 PSF

### SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

TOTAL WEIGHT = 2 X 38 = 76 lb

THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012, BCBC 2012, ABC 2014 - CSA 086-09

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.24")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.24") CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.20 (1-2:7), BC=0.06 (4-5:11), WB=0.08 (2-5:1) , SSI=0.10 (1-2:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.51 (3) (INPUT = 0.90) JSI METAL= 0.09 (1) (INPUT = 1.00)

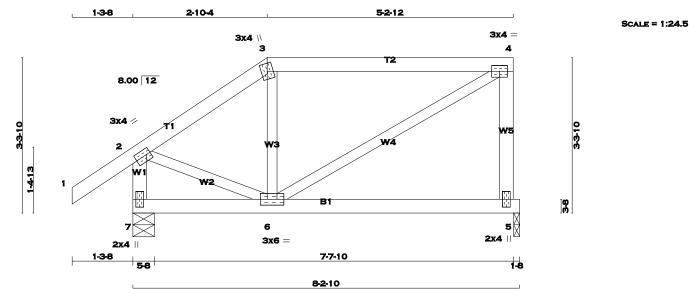




READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

POBPES GREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY DRWG NO. TW0317-048 T67 Kott Lumber Uxbridge, Stouffville, ON, TW

Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:21:05 2017 Page 1 ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-DspUXXuSXBI973dtBWAMJIPnO\_DMFPDhFtnpWwzcJJS



LUMBER				
N. L. G. A. R	ULES			
CHORDS	SIZE		LUMBER	DESCR.
1 - 3	2x4	DRY	No.2	SPF
3 - 4	2x4	DRY	No.2	SPF
5 - 4	2x4	DRY	No.2	SPF
7 - 2	2x4	DRY	No.2	SPF
7 - 5	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PL	PLATES (table is in inches)												
JT	TYPE	PLATES	W	LEN	Υ	Χ							
2	TMVW-t	MT20	3.0	4.0	1.50	1.00							
3	TTW+m	MT20	3.0	4.0									
4	TMVW-t	MT20	3.0	4.0									
5	BMV1+p	MT20	2.0	4.0									
6	BMWWW-t	MT20	3.0	6.0									
7	RM\/1+n	MT20	2.0	4 0									

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS	AND LOADINGS SPECIFIED	BY FABRICATOR	TO BE VERIFIED BY
BUILDING DESIGNER			

REAL	BEARINGS												
	FACTOR	ED	MAXIMUN	/ FACTO	INPUT	REQRD							
	<b>GROSS RE</b>	GROSS F	REACTIO	BRG	BRG								
JT	VERT	HORZ	DOWN	HORZ	<b>UPLIFT</b>	IN-SX	IN-SX						
5	383	0	409	0	-223	1-8	1-8						
7	490	0	520	191	-246	5-8	5-8						

PROVIDE ANCHORAGE AT BEARING JOINT 5 FOR 223 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 7 FOR 246 LBS FACTORED UPLIFT

PROVIDE FOR 191 LBS FACTORED HORIZONTAL REACTION AT JOINT 7

### UNFACTORED REACTIONS

	1ST LCASE	MAX./I	MIN. COMPON	NENT REACTION	JNS		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
5	269	188 / 0	0/0	0/0	65 / -211	81 / 0	0/0
7	341	252 / 0	0/0	0/0	76 / -233	89 / 0	0/0
HOR 7	IZONTAL REA	ACTIONS 0/0	0/0	0/0	136 / -98	0/0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 5, 7

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (11)

СНС	CHORDS				WEBS				
MAX.	FACTORED	FACTOR	RED				MAX. FACT	ORED	
MEMB.	FORCE	VERT. LO	AD LC	1 MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PL	F) (	CSI (LC)	<b>UNBRAC</b>		(LBS)	CSI (LC)	
FR-TO		FROM	TO		LENGTH	FR-TO			
1- 2	0 / 29	-77.3	-77.3	0.10(1)	10.00	6-3	-159 / 158	0.03 (7)	
2-3	-323 / 172	-77.3	-77.3	0.14 (7)	6.25	6- 4	-168 / 315	0.09 (7)	
3- 4	-272 / 203	-77.3	-77.3	0.37 (1)	6.25	2-6	-35 / 279	0.06(1)	
5- 4	-372 / 248	0.0	0.0	0.09 (7)	7.81				
7-2	-506 / 253	0.0	0.0	0.05(1)	7.81				
7- 6	-169 / 127	-17.5	-17.5	0.10 (11	6.25				
6- 5	-23 / 59	-17.5	-17.5	0.10 (11	6.25				

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF ( 9.0) PSE AT WIND LOAD APPLIED IS DERIVED FROM REPERSING VELOCITY FRESSURE OF (3.9), 25 AT (40-0.0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.

### **DESIGN CRITERIA**

SPEC	IFIED	LOAI	DS:		
TOP	CH.	LL	=	23.3	PSF
		DL	=	3.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.0	PSF
TOTA	I IO	AD	=	33.3	PSF

### SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

TOTAL WEIGHT = 34 lb

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THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

## THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09

- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.27\*) CALCULATED VERT. DEFL.(LL)= L/ 999 (0.00\*) ALLOWABLE DEFL.(TL)= L/360 (0.27\*) CALCULATED VERT. DEFL.(TL)= L/ 999 (0.02\*)

CSI: TC=0.37 (3-4:1), BC=0.10 (6-7:11), WB=0.09 (4-6:7), SSI=0.16 (3-4:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

### NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (2) (INPUT = 0.90 ) JSI METAL= 0.17 (2) (INPUT = 1.00 )





READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

JOB NAME TRUSS NAME QUANTITY PLY TW0317-048 **T68** 

Kott Lumber Uxbridge, Stouffville, ON, TW

POBPES GREENPARK-LECCO RIDGE-BLOCK 327 TRUSS DESC.

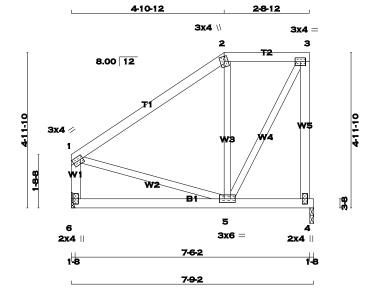
DRWG NO

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SCALE = 1:36.9

TOTAL WEIGHT = 37 lb

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LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR 2 2x4 2x4 DRY No.2 No.2 SPF SPF DRY 3 2x4 DRY No 2 SPF No.2 6 2x4 DRY No.2 SPF ALL WEBS DRY No.2 2x3 EXCEPT

DRY: SEASONED LUMBER

PLATES (table is in inches)

JΤ	TYPE	PLATES	W	LEN	Υ	Х
1	TMVW-t	MT20	3.0	4.0	1.50	1.00
2	TTW+m	MT20	3.0	4.0		
3	TMVW-t	MT20	3.0	4.0		
4	BMV1+p	MT20	2.0	4.0		
5	BMWWW-t	MT20	3.0	6.0		
6	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

BUILDING DESIGNER
BEARINGS DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

DEA	KINGS						
	FACTO	RED	MAXIMU	M FACT	INPUT	REQRD	
	GROSS R	EACTION	GROSS	REACTIO	BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
4	362	0	402	0	-228	1-8	1-8
6	362	0	384	260	-153	HANGER	BY OTHERS
						MIN. SEAT	Γ SIZE: 1-8

PROVIDE ANCHORAGE AT BEARING JOINT 4 FOR 228 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 6 FOR 153 LBS FACTORED UPLIFT

PROVIDE FOR 260 LBS FACTORED HORIZONTAL REACTION AT JOINT 6

UNFACTORED REACTIONS

ı	ONI ACTORED REACTIONS										
١		1ST LCASE	MAX./	MAX./MIN. COMPONENT REACTIONS							
١	JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL			
١	4	254	178 / 0	0/0	0/0	100 / -212	76 / 0	0/0			
١	6	254	178 / 0	0/0	0/0	55 / -158	76 / 0	0/0			
١											
١	HORIZONTAL REACTIONS										
١	6		0/0	0/0	0/0	185 / -134	0/0	0 /0			

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 4

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (11)

CHC	DRDS					WE	BS		
MAX.	FACTORED	FACTOR	RED				MAX. FACTO	DRED	
MEMB.	FORCE	VERT. LO	AD LC	1 MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PL	F) (	CSI (LC)	<b>UNBRAC</b>	)	(LBS)	CSI (LC)	
FR-TO		FROM	TO		LENGTH	FR-TO			
1- 2	-199 / 80	-77.3	-77.3	0.34 (7)	6.25	5- 2	-228 / 195	0.08(3)	
2-3	-160 / 174	-77.3	-77.3	0.09(7)	6.25	5-3	-170 / 323	0.07(7)	
4-3	-392 / 233	0.0	0.0	0.16 (7)	7.81	1- 5	-46 / 173	0.03(4)	
6- 1	-347 / 178	0.0	0.0	0.04 (7)	7.81				
6- 5	-232 / 175	-17.5	-17.5	0.09 (11	6.25				
5- 4	-36 / 92	-17.5	-17.5	0.09 (11	6.25				

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF ( 9.0) PSE AT WIND LOAD APPLIED IS DERIVED FROM REPERSING VELOCITY FRESSURE OF (3.9), 25 AT (40-0.0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.

### **DESIGN CRITERIA**

TOTAL LOAD

SPECIFIED LOADS: LL = DL = LL = DL = AD = CH. PSF 23.3 3.0 PSF PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

7.0 PSF

33.3 PSF

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09 - TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.25") CALCULATED VERT. DEFL.(LL)= L/ 999 (0.00") ALLOWABLE DEFL.(TL)= L/360 (0.25") CALCULATED VERT. DEFL.(TL)= L/999 (0.02")

CSI: TC=0.34 (1-2:7), BC=0.09 (4-5:11), WB=0.08 (2-5:3), SSI=0.13 (1-2:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

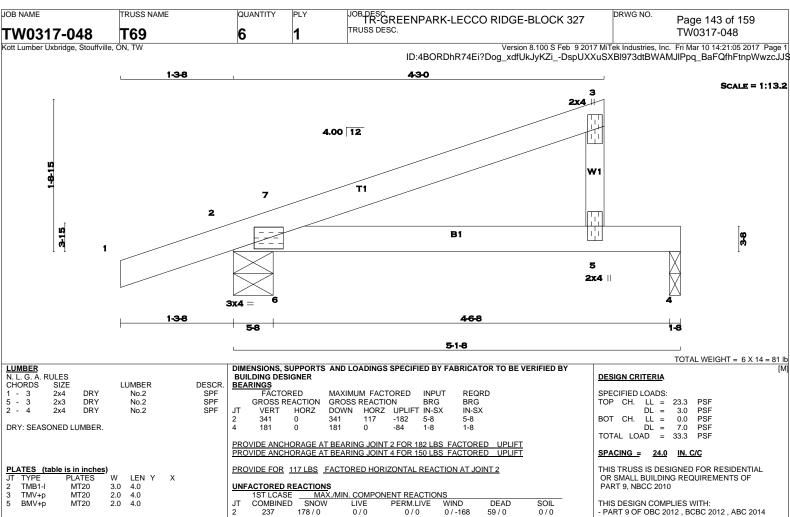
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.60 (5) (INPUT = 0.90) JSI METAL= 0.11 (1) (INPUT = 1.00)





READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

129 82 / 0 0/0 0/-90 46 / 0 0/0 HORIZONTAL REACTIONS 0/0 0/0 84 / 0 0 /0 0/0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 2, 4

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

<u>LOADING</u> TOTAL LOAD CASES: (11)

CHORDS				WEBS					
MAX.	FACTORED	FACTOR	RED				MAX. FACTO	DRED	
MEMB.	FORCE	VERT. LO.	AD LC	1 MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PL	F) (	CSI (LC)	UNBRAC		(LBS)	CSI (LC)	
FR-TO		FROM	TO		LENGTH	FR-TO			
1- 2	0 / 15	-77.3	-77.3	0.10(1)	10.00	6- 7	-71 / 165	0.00(1)	
2-7	-92 / 0	-77.3	-77.3	0.08 (11	) 6.25				
7-3	-48 / 4	-77.3	-77.3	0.21(1)	6.25				
5- 3	-154 / 117	0.0	0.0	0.05 (5)	7.81				
2-6	-16 / 33	-17.5	-175	0.11 (1)	6.25				
6-5	-16 / 33			0.11 (1)	6.25				
5- 4	0/0			0.21 (1)	10.00				

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF ( 9.0) PSE AT WIND LOAD APPLIED IS DERIVED FROM REPERSING VELOCITY FRESSURE OF (3.9), 25 AT (40-0.0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.

- CSA 086-09 TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19") CALCULATED VERT. DEFL.(LL) = L/999 (0.05") ALLOWABLE DEFL.(TL)= L/360 (0.19") CALCULATED VERT. DEFL.(TL) = L/694 (0.09")

CSI: TC=0.21 (3-7:1), BC=0.21 (5-6:1), WB=0.00 (6-7:1), SSI=0.14 (4-5:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

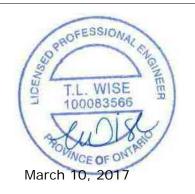
NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION

(PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (2) (INPUT = 0.90 ) JSI METAL= 0.05 (2) (INPUT = 1.00 )





READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

POBPES GREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY PLY TW0317-048 T70 Kott Lumber Uxbridge, Stouffville, ON, TW

DRWG NO.

ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-h2Nsltv4IVt0IDC4IDhbryxuiOXW\_ndrUXXN2MzcJJR

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1.3-8 5-7-13 11-8-7 5-1-13 SCALE = 1:41.0 4x6 = 5x5 // 2x4 || 3 8.00 12 4x4 = 4x5 🗸 团 WS 9 11 10 8 **3**x6 = 3x6 = 3x8 = 3x5 = 2x4 || 22-0-8 1-10 22-7-10

LUMBER				
N. L. G. A. R	ULES			
CHORDS	SIZE		LUMBER	DESCR.
1 - 3	2x4	DRY	No.2	SPF
3 - 5	2x4	DRY	No.2	SPF
5 - 6	2x4	DRY	No.2	SPF
12 - 2	2x4	DRY	No.2	SPF
7 - 6	2x4	DRY	No.2	SPF
12 - 9	2x4	DRY	No.2	SPF
9 - 7	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)									
JT	TYPE	PLATES	W	LEN	Υ	X			
2	TMVW-t	MT20	4.0	5.0	1.75	2.00			
3	TTWW-m	MT20	4.0	6.0	1.75	1.50			
4	TMW+w	MT20	2.0	4.0					
5	TTWW+m	MT20	5.0	5.0	2.25	1.25			
6	TMVW-p	MT20	4.0	4.0	1.25	2.00			
7	BMV1+p	MT20	2.0	4.0					
8	BMWW-t	MT20	3.0	5.0	1.50	2.25			
9	BS-t	MT20	3.0	6.0					
10	BMWWW-t	MT20	3.0	8.0					
11	BMWW-t	MT20	3.0	6.0	1.50	2.25			
12	BMV1+p	MT20	2.0	4.0					

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

## DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

DEA	KINGS						
	FACTOR	ED	MAXIMUN	M FACTO	ORED	INPUT	REQRD
	<b>GROSS RE</b>	ACTION	GROSS F	REACTIO	N	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
12	1173	0	1247	241	-596	5-8	5-8
7	1067	0	1101	0	-533	1-10	1-10

PROVIDE ANCHORAGE AT BEARING JOINT 12 FOR 596 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 7 FOR 533 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES

PROVIDE FOR 241 LBS FACTORED HORIZONTAL REACTION AT JOINT 12

UNFACTORED REACTIONS									
	1ST LCASE	MAX./	MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
12	821	588 / 0	0/0	0/0	183 / -576	233 / 0	0/0		
7	749	524 / 0	0/0	0/0	86 / -525	225 / 0	0/0		
HORIZONTAL REACTIONS									
12		0/0	0/0	0/0	172 / -158	0/0	0 /0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 12, 7

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.99 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (11)

	CHORDS WEBS							
MAX	(. FACTORED	FACTO	RED				MAX. FACTO	DRED
MEMB.	FORCE	VERT. LO	AD LC	1 MAX	MAX.	MEMB.	FORCE	MAX
	(LBS)	(PL	_F) (	CSI (LC)	<b>UNBRAC</b>	2	(LBS)	CSI (LC)
FR-TO		FROM	TO		LENGTH	FR-TO		
1- 2	0/29	-77.3	-77.3	0.10(1)	10.00	11- 3	-66 / 144	0.03 (8)
2-3	-1198 / 591	-77.3	-77.3	0.52 (7)	5.20	3-10	-325 / 481	0.36 (8)
3- 4	-1374 / 794	-77.3	-77.3	0.50(1)	4.99	10- 4	-584 / 451	0.22(3)
4- 5	-1374 / 794	-77.3	-77.3	0.50(1)	4.99	10- 5	-364 / 590	0.40 (7)
5-6	-1113 / 559	-77.3	-77.3	0.44 (8)	5.55	8- 5	-139 / 170	0.05(3)
12-2	-1206 / 625	0.0		0.12(1)	7.33	2-11	-341 / 980	0.22(1)
7- 6	-1066 / 558	0.0	0.0	0.11 (1)	7.67	8- 6	-357 / 959	0.21 (1)
12-11	-220 / 212	-17.5	-17.5	0.14 (11)	6.25			
11-10	-417 / 1016	-17.5	-17.5	0.23 (1)	6.25			
10-9	-314 / 923	-17.5	-17.5	0.21(1)	6.25			
9-8	-314 / 923	-17.5	-17.5	0.21(1)	6.25			
8- 7	-13 / 28	-17.5	-17.5	0.13 (11)	6.25			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM],INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

**RECEIVED** TOWN OF MILTON MAR 29, 2017 17-4978 **BUILDING DIVISION** 

# **DESIGN CRITERIA**

SPECIFIED LOADS: LL = DL = LL = DL = AD = CH. PSF 3.0 PSF 7.0 PSF TOTAL LOAD 33.3

### SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

TOTAL WEIGHT = 90 lb

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09

- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.75")
CALCULATED VERT. DEFL.(LL)= L/999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.75")
CALCULATED VERT. DEFL.(TL)= L/999 (0.08")

CSI: TC=0.52 (2-3:7) , BC=0.23 (10-11:1) , WB=0.40 (5-10:7) , SSI=0.22 (3-4:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (6) (INPUT = 0.90 ) JSI METAL= 0.41 (2) (INPUT = 1.00) JOB NAME TW0317-048 QUANTITY

PLY

POBPES GREENPARK-LECCO RIDGE-BLOCK 327

DRWG NO.

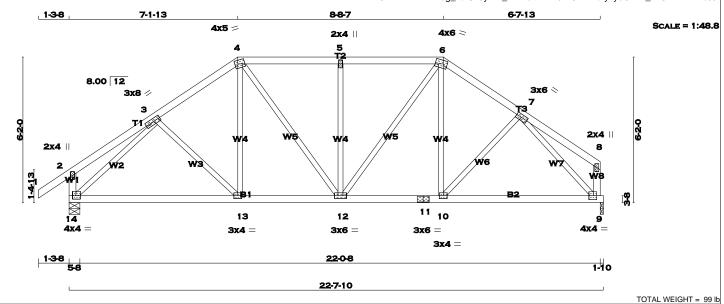
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Kott Lumber Uxbridge, Stouffville, ON, TW

TRUSS NAME

T71

Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:21:06 2017 Page 1 ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-h2Nsltv4IVt0IDC4IDhbryxyoOWw\_lvrUXXN2MzcJJR



LUMBER				
N. L. G. A. R	ULES			
CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 6	2x4	DRY	No.2	SPF
6 - 8	2x4	DRY	No.2	SPF
14 - 2	2x4	DRY	No.2	SPF
9 - 8	2x4	DRY	No.2	SPF
14 - 11	2x4	DRY	No.2	SPF
11 - 9	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PL/	ATES	(table is in inches)	
JT	TYPE	PLATES	W

JT	TYPE	PLATES	W	LEN	Υ	Χ	
2	TMV+p	MT20	2.0	4.0			
3	TMWW-t	MT20	3.0	8.0	1.50	3.50	
4	TTWW-m	MT20	4.0	5.0	1.75	1.50	
5	TMW+w	MT20	2.0	4.0			
6	TTWW-m	MT20	4.0	6.0	1.75	2.00	
7	TMWW-t	MT20	3.0	6.0	1.50	2.75	
8	TMV+p	MT20	2.0	4.0			
9	BMVW1-t	MT20	4.0	4.0	2.00	1.75	
10	BMWW-t	MT20	3.0	4.0			
11	BS-t	MT20	3.0	6.0			
12	BMWWW-t	MT20	3.0	6.0			
13	BMWW-t	MT20	3.0	4.0			
14	BMVW1-t	MT20	4.0	4.0	1.75	1.75	

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH

## DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

3EAI	RINGS						
	FACTOR	ED	MAXIMUN	/ FACTO	ORED	INPUT	REQRD
	<b>GROSS RE</b>	ACTION	GROSS F	REACTIO	N	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	<b>UPLIFT</b>	IN-SX	IN-SX
14	1173	0	1249	281	-580	5-8	5-8
9	1067	0	1110	0	-515	1-10	1-10

PROVIDE ANCHORAGE AT BEARING JOINT 14 FOR 580 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 9 FOR 515 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES

PROVIDE FOR 281 LBS FACTORED HORIZONTAL REACTION AT JOINT 14

### UNFACTORED REACTIONS

	1ST LCASE	MAX./	MIN. COMPO	<u>NENT REACTIO</u>	ONS		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
14	821	588 / 0	0/0	0/0	189 / -564	233 / 0	0/0
9	749	524 / 0	0/0	0/0	106 / -513	225 / 0	0/0
HOR	IZONTAL REA	ACTIONS					
14		0/0	0/0	0/0	200 / -186	0/0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 14, 9

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.79 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (11)

СН	IORDS	WEBS						
MA.	X. FACTORED	FACTOR	RED				MAX. FACTO	ORED
MEMB.	FORCE	VERT. LO.	AD LC	1 MAX	MAX.	MEMB	. FORCE	MAX
	(LBS)	(PL	F) (	CSI (LC)	<b>UNBRAC</b>	2	(LBS)	CSI (LC)
FR-TO		FROM	TO		LENGTH	FR-TO	)	
1- 2	0 / 29	-77.3	-77.3	0.10(1)	10.00	3-13	-94 / 251	0.05 (7)
2-3	-25 / 159	-77.3	-77.3	0.19 (7)	6.25	13- 4	-75 / 196	0.04 (7)
3- 4	-1175 / 649	-77.3	-77.3	0.26 (7)	5.79	4-12	-235 / 266	0.23 (8)
4- 5	-1113 / 674	-77.3	-77.3	0.26 (7)	5.85	12- 5	-430 / 332	0.25(3)
5-6	-1113 / 674	-77.3	-77.3	0.26 (7)	5.85	12-6	-264 / 348	0.26 (7)
6- 7	-1105 / 626			0.23 (8)		10-6	-46 / 134	0.03 (11)
7-8	-23 / 104			0.16 (8)		10- 7	-50 / 188	0.04 (8)
14- 2	-248 / 237	0.0	0.0	0.04 (7)	7.81	14- 3	-1362 / 545	0.51 (1)
9- 8	-128 / 137	0.0	0.0	0.03 (8)	7.81	7- 9	-1292 / 560	0.48 (1)
14-13	-544 / 1044	-17.5	-17.5	0.26 (11	) 6.25			
13-12	-362 / 978			0.27 (11				
12-11	-275 / 909	-17.5	-17.5	0.24 (11	) 6.25			
11-10	-275 / 909	-17.5	-17.5	0.24 (11	) 6.25			
10-9	-350 / 873	-17.5	-17.5	0.23 (1)	6.25			
1								

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF ( 9.0) PSF AT WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF § 9.0 PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2), BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

**RECEIVED** TOWN OF MILTON MAR 29, 2017 17-4978 **BUILDING DIVISION** 

# **DESIGN CRITERIA**

SPECIFIED LOADS: LL = 23.3 DL = 3.0 LL = 0.0 DL = 7.0 AD = 33.3 CH. PSF PSF PSF TOTAL LOAD PSF

### SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09

- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.75") CALCULATED VERT. DEFL.(LL)= L/ 999 (0.04") ALLOWABLE DEFL.(TL)= L/360 (0.75") CALCULATED VERT. DEFL.(TL)= L/ 999 (0.10")

CSI: TC=0.26 (4-5:7) , BC=0.27 (12-13:11) , WB=0.51 (3-14:1) , SSI=0.16 (4-5:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

### NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

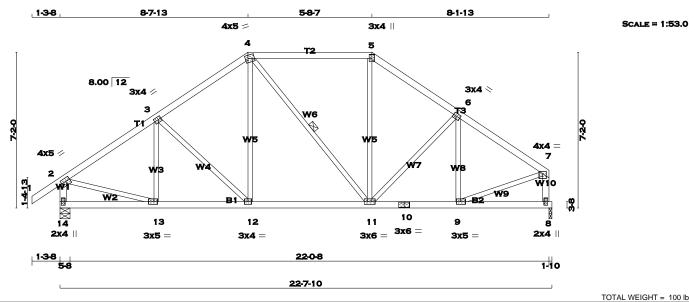
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (4) (INPUT = 0.90) JSI METAL= 0.36 (14) (INPUT = 1.00)



JOB PESC TR-GREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY DRWG NO Page 146 of 159 TW0317-048 TW0317-048 T72 Kott Lumber Uxbridge, Stouffville, ON, TW

Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:21:07 2017 Page 1 ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-9ExFyDwi3p?tNNnGJxCqOAU6fos6jHb\_jBGwapzcJJQ

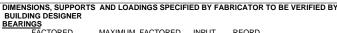


<u>LUMBER</u>				
N. L. G. A. R	ULES			
CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 5	2x4	DRY	No.2	SPF
5 - 7	2x4	DRY	No.2	SPF
14 - 2	2x4	DRY	No.2	SPF
8 - 7	2x4	DRY	No.2	SPF
14 - 10	2x4	DRY	No.2	SPF
10 - 8	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)											
JT	TYPE	PLATES	W	LEN	Υ	Χ					
2	TMVW-t	MT20	4.0	5.0	1.75	2.00					
3	TMWW-t	MT20	3.0	4.0	1.50	1.50					
4	TTWW-m	MT20	4.0	5.0	1.75	1.50					
5	TTW+p	MT20	3.0	4.0	2.25	1.50					
6	TMWW-t	MT20	3.0	4.0	1.50	1.50					
7	TMVW-p	MT20	4.0	4.0	1.25	2.00					
8	BMV1+p	MT20	2.0	4.0							
9	BMWW-t	MT20	3.0	5.0	1.50	2.25					
10	BS-t	MT20	3.0	6.0							
11	BMWWW-t	MT20	3.0	6.0							
12	BMWW-t	MT20	3.0	4.0							
13	BMWW-t	MT20	3.0	5.0	1.50	2.00					
14	BMV1+p	MT20	2.0	4.0							

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH



	FACTOR GROSS RE		MAXIMUI GROSS F		INPUT BRG	REQRD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
14	1173	0	1250	320	-560	5-8	5-8
8	1067	0	1119	0	-494	1-10	1-10

PROVIDE ANCHORAGE AT BEARING JOINT 14 FOR 560 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 8 FOR 494 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES

PROVIDE FOR 320 LBS FACTORED HORIZONTAL REACTION AT JOINT 14

UNF	UNFACTORED REACTIONS										
	1ST LCASE	MAX./	MIN. COMPO	NENT REACTION	ONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL				
14	821	588 / 0	0/0	0/0	191 / -550	233 / 0	0/0				
8	749	524 / 0	0/0	0/0	130 / -498	225 / 0	0/0				
HORIZONTAL REACTIONS 14 0/0 0/0 0/0 229/-214 0/0 0/0											
RΕΔΙ	READING MATERIAL TO BE SPENIO 2 OR RETTER AT IOINT/S) 14 8										

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 4-11. DBS = 20-0-0 . CBF = 14 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING TOTAL LOAD CASES: (11)

	C H O R D S W E B S MAX. FACTORED FACTORED MAX. FACTORED									
MEMB.				1 MAY	MAY	MEMB				
IVILIVID.	(LBS)			CSI (LC)			(LBS)			
FD TO							(LDS)	CSI (LC)		
FR-TO		FROM T			LENGTH					
1- 2	0 / 29			0.10(1)			-173 / 146	0.05 (1)		
2- 3	-1245 / 589	-77.3	-77.3	0.33(7)	5.58	3-12	-277 / 304	0.16 (3)		
3- 4	-1092 / 614	-77.3	-77.3	0.34 (7)	5.86	12-4	-145 / 278	0.13 (7)		
4- 5	-862 / 569	-77.3	-77.3	0.38 (8)	6.24	4-11	-132 / 138	0.07 (5)		
5- 6	-1052 / 601	-77.3	-77.3	0.31 (8)	5.99	11-5	-87 / 228	0.08 (7)		
6- 7	-1118 / 537	-77.3	-77.3	0.30 (8)	5.83	11-6	-193 / 252	0.11 (4)		
14- 2	-1215 / 581	0.0	0.0	0.12(1)	7.30	9-6	-247 / 174	0.07 (1)		
8- 7	-1086 / 515	0.0	0.0	0.11 (1)	7.61	2-13	-353 / 1059	0.23 (1)		
						9- 7	-364 / 995	0.21 (1)		
14-13	-299 / 290	-17.5	-17.5	0.07 (11	) 6.25			. ,		
13-12	-522 / 1106	-17.5	-17.5	0.21 (1)	6.25					
12-11	-302 / 908	-17.5	-17.5	0.18(1)	6.25					
11-10	-318 / 942	-17.5	-17.5	0.20(1)	6.25					
10-9	-318 / 942	-17.5	-17.5	0.20(1)	6.25					
9-8	<u>-</u> 13/28	-17.5	-17.5	0.06 (11	) 6.25					

ERENCE VELOCITY PRESSURE OF ( 9.0) PSF AT THE NOTE PAGE COEL FLORM'S, CACY, BASED ON THE (MAIN WIND FORCE RESIS WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

17-4978

**BUILDING DIVISION** 

**DESIGN CRITERIA** 

SPECIFIED LOADS: LL = DL = LL = DL = AD = CH. PSF 3.0 PSF 7.0 PSF TOTAL LOAD 33.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09 - TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.75") CALCULATED VERT. DEFL.(LL)= L/ 999 (0.03") ALLOWABLE DEFL.(TL)= L/360 (0.75") CALCULATED VERT. DEFL.(TL)= L/ 999 (0.07")

CSI: TC=0.38 (4-5:8), BC=0.21 (12-13:1), WB=0.23 (2-13:1), SSI=0.17 (4-5:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

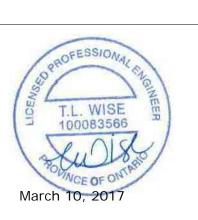
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

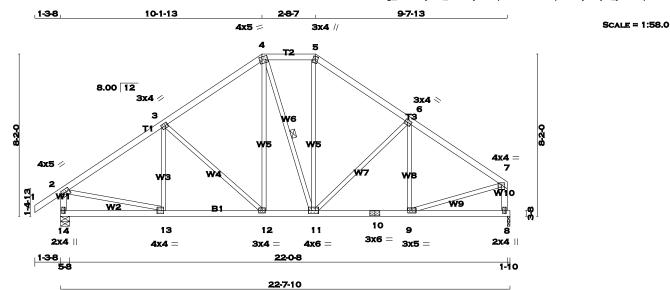
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (5) (INPUT = 0.90 ) JSI METAL= 0.42 (2) (INPUT = 1.00 )



JOB PESC TR-GREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY PLY DRWG NO Page 147 of 159 TW0317-048 TW0317-048 T73

Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:21:07 2017 Page 1 Kott Lumber Uxbridge, Stouffville, ON, TW ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-9ExFyDwi3p?tNNnGJxCqOAU5yoszjFr\_jBGwapzcJJQ



LUMBER				
N. L. G. A. R	ULES			
CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 5	2x4	DRY	No.2	SPF
5 - 7	2x4	DRY	No.2	SPF
14 - 2	2x4	DRY	No.2	SPF
8 - 7	2x4	DRY	No.2	SPF
14 - 10	2x4	DRY	No.2	SPF
10 - 8	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PL/	PLATES (table is in inches)											
JT	TYPE	PLATES	· W	LEN	Υ	Χ						
2	TMVW-t	MT20	4.0	5.0	1.75	2.00						
3	TMWW-t	MT20	3.0	4.0	1.50	1.50						
4	TTWW-m	MT20	4.0	5.0	1.75	1.50						
5	TTW+m	MT20	3.0	4.0	2.00	1.25						
6	TMWW-t	MT20	3.0	4.0	1.50	1.50						
7	TMVW-p	MT20	4.0	4.0	1.25	2.00						
8	BMV1+p	MT20	2.0	4.0								
9	BMWW-t	MT20	3.0	5.0	1.50	2.25						
10	BS-t	MT20	3.0	6.0								
11	BMWWW-t	MT20	4.0	6.0								
12	BMWW-t	MT20	3.0	4.0								
13	BMWW-t	MT20	4.0	4.0	2.00	1.50						
14	BMV1+n	MT20	2.0	4.0								

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

# DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS

	FACTOR GROSS RE		MAXIMUI GROSS I		INPUT BRG	REQRD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
14	1173	0	1249	359	-535	5-8	5-8
8	1067	0	1127	0	-469	1-10	1-10

PROVIDE ANCHORAGE AT BEARING JOINT 14 FOR 535 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 8 FOR 469 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES

PROVIDE FOR 359 LBS FACTORED HORIZONTAL REACTION AT JOINT 14

UNF	JNFACTORED REACTIONS										
	1ST LCASE	MAX./	MIN. COMPO	NENT REACTION	ONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL				
14	821	588 / 0	0/0	0/0	189 / -532	233 / 0	0/0				
8	749	524 / 0	0/0	0/0	149 / -480	225 / 0	0/0				
HOR	IZONTAL RE	ACTIONS									
14		0/0	0/0	0/0	257 / -242	0/0	0 /0				
	DINO MATER	TO DE	2DE NO 2 OD	DETTED AT 10	NINT(O) 44 C						

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 14, 8

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.46 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 4-11. DBS = 20-0-0 . CBF = 16 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING TOTAL LOAD CASES: (11)

	ORDS X. FACTORED	FACTOR	RED		W E B S MAX. FACTORED			
MEMB.		VERT. LO	AD LC	1 MAX	MAX.	MEMB.		
	(LBS)			CSI (LC)	UNBRAC		(LBS)	CSI (LC)
FR-TO	, ,	FROM '	ΤΌ	, ,	LENGTH	FR-TO	, ,	` ,
1- 2	0 / 29	-77.3	-77.3	0.10(1)	10.00	13-3	-115 / 144	0.04 (4)
2- 3	-1252 / 557	-77.3	-77.3	0.43 (7)	5.46	3-12	-404 / 385	0.34 (3)
3- 4	-996 / 555	-77.3	-77.3	0.42 (7)	5.96	12-4	-209 / 328	0.27 (7)
4- 5	-804 / 541	-77.3	-77.3	0.14 (8)	6.25	4-11	-145 / 147	0.06 (5)
5- 6	-988 / 558	-77.3	-77.3	0.39 (8)	6.04	11- 5	-163 / 290	0.21 (7)
6- 7	-1149 / 515	-77.3	-77.3	0.39 (8)	5.68	11-6	-321 / 336	0.26 (4)
14- 2	-1211 / 560	0.0	0.0	0.12(1)	7.32	9-6	-184 / 166	0.07(3)
8- 7	-1091 / 492	0.0	0.0	0.11 (1)	7.60	2-13	-322 / 1064	0.23 (1)
						9- 7	-330 / 1010	0.22 (1)
14-13	-338 / 329	-17.5	-17.5	0.12 (11)	6.25			
13-12	-508 / 1125	-17.5	-17.5	0.22 (1)	6.25			
12-11	-224 / 828	-17.5	-17.5	0.15 (1)	6.25			
11-10	-290 / 971			0.20(1)				
10- 9	-290 / 971	-17.5	-17.5	0.20(1)	6.25			
9-8	-13/28	-17.5	-17.5	0.10 (11)	6.25			
			7.					

ERENCE VELOCITY PRESSURE OF ( 9.0) PSF AT THE NOTE PAGE COEL FLORM'S, CACY, BASED ON THE (MAIN WIND FORCE RESIS WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

17-4978

**BUILDING DIVISION** 

**DESIGN CRITERIA** 

SPECIFIED LOADS: LL = 23.3 DL = 3.0 LL = 0.0 DL = 7.0 AD = 33.3 CH. PSF PSF PSF TOTAL LOAD PSF

### SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

TOTAL WEIGHT = 105 lb

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09 - TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.75") CALCULATED VERT. DEFL.(LL)= L/ 999 (0.04") ALLOWABLE DEFL.(TL)= L/360 (0.75") CALCULATED VERT. DEFL.(TL)= L/999 (0.07")

CSI: TC=0.43 (2-3:7), BC=0.22 (12-13:1), WB=0.34 (3-12:3), SSI=0.17 (2-3:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

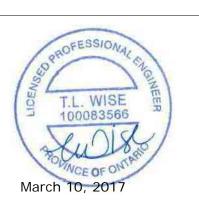
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (7) (INPUT = 0.90) JSI METAL= 0.43 (2) (INPUT = 1.00)



JOB NAME TRUSS NAME QUANTITY PLY TRUSS DESC TW0317-048 T74 Kott Lumber Uxbridge, Stouffville, ON, TW

JOB PESC PREENPARK-LECCO RIDGE-BLOCK 327

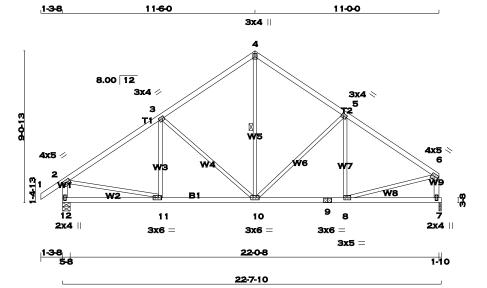
DRWG NO

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SCALE = 1:68.8

TOTAL WEIGHT = 97 lb

Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:21:08 2017 Page 1 ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-eRVdAYxLp67k?XMStej3xN1EECC\_Sfh8yr0T7FzcJJP



LUMBER N. L. G. A. CHORDS LUMBER DESCR SIZE 2x4 DRY No.2 No.2 SPF SPF 2x4 DRY 12 -7 -12 -2x4 DRY No 2 SPF 2x4 No.2 9 2x4 DRY No.2 SPF DRY No.2 SPF ALL WERS 2x3 DRY No.2 SPF EXCEPT

DRY: SEASONED LUMBER

DI ATES (table is in inches)

1 1/	LATES (table is in inches)										
JT	TYPE	PLATES	W	LEN	Υ	X					
2	TMVW-t	MT20	4.0	5.0	1.75	2.00					
3	TMWW-t	MT20	3.0	4.0	1.50	1.50					
4	TTW+p	MT20	3.0	4.0	2.25	1.50					
5	TMWW-t	MT20	3.0	4.0	1.50	1.50					
6	TMVW-t	MT20	4.0	5.0	1.75	Edge					
7	BMV1+p	MT20	2.0	4.0							
8	BMWW-t	MT20	3.0	5.0	1.50	2.25					
9	BS-t	MT20	3.0	6.0							
10	BMWWW-t	MT20	3.0	6.0							
11	BMWW-t	MT20	3.0	6.0	1.50	2.00					
12	BMV1+p	MT20	2.0	4.0							

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

### DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS											
FACTOR	ED	MAXIMUM FACTORED			INPUT	REQRD					
GROSS RE	ACTION	GROSS F	REACTIO	BRG	BRG						
VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX					
1173	0	1247	394	-511	5-8	5-8					
1067	0	1132	0	-444	1-10	1-10					
	FACTOR GROSS RE VERT 1173	FACTORED GROSS REACTION VERT HORZ 1173 0	FACTORED MAXIMUI GROSS REACTION GROSS F VERT HORZ DOWN 1173 0 1247	FACTORED MAXIMUM FACTOR GROSS REACTION GROSS REACTION VERT HORZ DOWN HORZ 1173 0 1247 394	FACTORED MAXIMUM FACTORED GROSS REACTION GROSS REACTION VERT HORZ DOWN HORZ UPLIFT 1173 0 1247 394 -511	FACTORED MAXIMUM FACTORED INPUT GROSS REACTION BRG VERT HORZ DOWN HORZ UPLIFT IN-SX 1173 0 1247 394 -511 5-8					

PROVIDE ANCHORAGE AT BEARING JOINT 12 FOR 511 LBS FACTORED PROVIDE ANCHORAGE AT BEARING JOINT 7 FOR 444 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES

PROVIDE FOR 394 LBS FACTORED HORIZONTAL REACTION AT JOINT 12

UNFACTORED REACTIONS

	1ST LCASE MAX./MIN. COMPONENT REACTIONS								
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
12	821	588 / 0	0/0	0/0	183 / -515	233 / 0	0/0		
7	749	524 / 0	0/0	0/0	163 / -462	225 / 0	0/0		
HORIZONTAL REACTIONS 12 0/0 0/0 0/0 281/-267 0/0 0/0									

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 12, 7

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.36 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 4-10. DBS = 20-0-0 . CBF = 40 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING TOTAL LOAD CASES: (11)

	CH	CHORDS				WEBS				
MAX. FACTORED FACTORED			ED				MAX. FACTO	RED		
	MEMB.	FORCE	VERT. LOA	ND LC1	I MAX	MAX.	MEMB.	FORCE	MAX	
		(LBS)	(PLF	•) (	CSI (LC)	<b>UNBRAC</b>		(LBS)	CSI (LC)	
	FR-TO		FROM T	Ō		LENGTH	FR-TO			
	1- 2	0 / 29	-77.3	-77.3	0.10(1)	10.00	11-3	-83 / 140	0.03(4)	
	2- 3	-1243 / 519	-77.3	-77.3	0.52 (7)	5.36	3-10	-501 / 458	0.56 (3)	
	3- 4	-929 / 532	-77.3	-77.3	0.52 (7)	6.03	10- 4	-370 / 647	0.19 (7)	
	4- 5	-929 / 540	-77.3	-77.3	0.48 (8)	6.09	10- 5	-422 / 414	0.46 (4)	
	5- 6	-1161 / 484	-77.3	-77.3	0.48 (8)	5.55	8- 5	-143 / 158	0.06(3)	
	12- 2	-1204 / 539	0.0	0.0	0.12(1)	7.33	2-11	-306 / 1074	0.23(1)	
	7- 6	-1091 / 471	0.0	0.0	0.11(1)	7.60	8-6	-317 / 1017	0.22(1)	
	12-11	-373 / 364	-17.5	-17.5	0.14 (11	) 6.25				
	11-10	-488 / 1129			0.23 (1)					
	10-9	-280 / 986			0.21 (1)					
	9-8	-280 / 986	-17.5	-17.5	0.21(1)	6.25				
	8- 7	-13 / 28	-17.5	-17.5	0.13 (11	) 6.25				

T.L. WISE TO 100083566 100083566 WCE OF ON

March 10, 2017

TE GRADE AND USING EXTERNAL PEAK
I WIND FORCE RESISTING SYSTEM INTERNAL
TEGORY 2, BUILDING MAY BE 100 CATED ON
TO BE LOCATED AT LEAST (REPETED VIEW)

RENCE VELOCITY PRESSURE OF { 9.0} PSF AT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

TOWN OF MILTON MAR 29, 2017 17-4978 **BUILDING DIVISION** 

### **DESIGN CRITERIA**

SPECIFIED LOADS: LL = DL = LL = DL = AD = CH. PSF 3.0 PSF 7.0 PSF

### SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

33.3 PSF

THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012, BCBC 2012, ABC 2014

- CSA 086-09 - TPIC 2011

TOTAL LOAD

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.75") CALCULATED VERT. DEFL.(LL) = L/999 (0.04") ALLOWABLE DEFL.(TL)= L/360 (0.75") CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.52 (2-3:7) , BC=0.23 (10-11:1) , WB=0.56 (3-10:3) , SSI=0.19 (2-3:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION

(PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (8) (INPUT = 0.90 ) JSI METAL= 0.43 (2) (INPUT = 1.00 )

JOB NAME TRUSS NAME

Kott Lumber Uxbridge, Stouffville, ON, TW

T75

TW0317-048

QUANTITY PLY 2

1.3-8

660

JOB PR-GREENPARK-LECCO RIDGE-BLOCK 327 TRUSS DESC

DRWG NO.

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SCALE = 1:42.2

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3x4 II 8.00 12 3x4 / 3x4 < 5 3 2x4 || 2x4 || 6 2 8 3x6 = 3x4 = 1.3-8 12:3-0

LUMBER	LUMBER							
N. L. G. A. RULES								
CHORDS	SIZE		LUMBER DE					
1 - 4	2x4	DRY	No.2	SPF				
4 - 6	2x4	DRY	No.2	SPF				
9 - 2	2x4	DRY	No.2	SPF				
7 - 6	2x4	DRY	No.2	SPF				
9 - 7	2x4	DRY	No.2	SPF				
ALL WEBS	2x3	DRY	No.2	SPF				
EXCEPT								

DRY: SEASONED LUMBER.

JΤ	TYPE	PLATES	W	LEN	Υ	Х	
2	TMV+p	MT20	2.0	4.0			
3	TMWW-t	MT20	3.0	4.0	1.50	1.75	
4	TTW+p	MT20	3.0	4.0	2.25	1.50	
5	TMWW-t	MT20	3.0	4.0	1.50	1.75	
6	TMV+p	MT20	2.0	4.0			
7	BMVW1-t	MT20	3.0	4.0			
8	BMWWW-t	MT20	3.0	6.0			
9	BMVW1-t	MT20	3.0	4.0			

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

### DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY **BUILDING DESIGNER**

1300

	FACTOR	RED	MAXIMUM FACTORED			INPUT	REQRD
	<b>GROSS RE</b>	GROSS REACTION			BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
9	723	0	764	256	-315	5-8	5-8
7	617	0	653	0	-256	HANGER B	Y OTHERS
						MIN. SEAT	SIZE: 3-8

PROVIDE ANCHORAGE AT BEARING JOINT 9 FOR 315 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 7 FOR 256 LBS FACTORED UPLIFT

PROVIDE FOR 256 LBS FACTORED HORIZONTAL REACTION AT JOINT 9

## UNFACTORED REACTIONS

	CHI ACTORED REACTIONS										
	1ST LCASE	MAX./	MAX./MIN. COMPONENT REACTIONS								
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL				
9	505	367 / 0	0/0	0/0	104 / -314	138 / 0	0/0				
7	433	303 / 0	0/0	0/0	91 / -267	130 / 0	0/0				
HORIZONTAL REACTIONS											
9		0/0	0/0	0/0	183 / -174	0/0	0 /0				

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 9

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (11)

СНО	CHORDS					WEBS				
MAX.	FACTORED	FACTO	RED				MAX. FACTO	DRED		
MEMB.	FORCE	VERT. LO	AD LC	1 MAX	MAX.	MEMB.	FORCE	MAX		
	(LBS)	(PL	F) (	CSI (LC)	UNBRAC		(LBS)	CSI (LC)		
FR-TO	, ,	FROM	ΤΌ	. ,	LENGTH	FR-TO	. ,	, ,		
1-2	0 / 29	-77.3	-77.3	0.10(1)	10.00	8- 4	-193 / 344	0.09 (7)		
2-3	-24 / 151	-77.3	-77.3	0.16 (7)	6.25	8- 5	-175 / 272	0.06 (4)		
3- 4	-518 / 294	-77.3	-77.3	0.18 (7)	6.25	3-8	-175 / 283	0.06(3)		
4- 5	-519 / 297	-77.3	-77.3	0.18 (8)	6.25	9-3	-718 / 221	0.23 (4)		
5-6	-20 / 94	-77.3	-77.3	0.15 (8)	6.25	5-7	-711 / 251	0.22 (3)		
9- 2	-235 / 225	0.0	0.0	0.04 (7)	7.81					
7-6	-122 / 129	0.0	0.0	0.02 (8)	7.81					
9-8	-260 / 546	-17.5	-17.5	0.24 (11	6.25					
8- 7	-156 / 503	-17.5	-17.5	0.24 (11	6.25					

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF ( 9.0) PSF AT WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (\$3.0) PSP AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.

# **DESIGN CRITERIA**

SPEC	IFIED	LOAI	DS:		
TOP	CH.	LL	=	23.3	PSF
		DI	_	3.0	PSF

DL 7.0 PSF TOTAL LOAD 33.3 PSF

### SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

TOTAL WEIGHT = 2 X 55 = 110 lb

## THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014 - CSA 086-09

TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43") CALCULATED VERT. DEFL.(LL) = L/999 (0.01") ALLOWABLE DEFL.(TL)= L/360 (0.43") CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.18 (3-4:7) , BC=0.24 (8-9:11) , WB=0.23 (3-9:4) , SSI=0.11 (4-5:8)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

## NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (7) (INPUT = 0.90 ) JSI METAL= 0.26 (5) (INPUT = 1.00 )





READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

JOB NAME TRUSS NAME TW0317-048 T76

Kott Lumber Uxbridge, Stouffville, ON, TW

QUANTITY 2

5-11-8

1-3-8

JOB PESC TR-GREENPARK-LECCO RIDGE-BLOCK 327 TRUSS DESC.

DRWG NO

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SCALE = 1:39.7

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1-3-8

1.3-8 5-8

3 8.00 12 3x5 / 3x5 < 2 W2 W2 3x6 2x4 || 2x4 || 1.3-8

3x4 II

LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR 3 2x4 2x4 DRY No.2 No.2 SPF SPF DRY 2x4 DRY No 2 SPF 2x4 No.2 8 6 2x4 DRY No.2 SPF ALL WEBS DRY No.2 SPF 2x3 EXCEPT

DRY: SEASONED LUMBER

PL/	PLATES (table is in inches)										
JT	TYPE	PLATES	W	LEN	Υ	X					
2	TMVW-t	MT20	3.0	5.0	1.50	2.00					
3	TTW+p	MT20	3.0	4.0	2.25	1.50					
4	TMVW-t	MT20	3.0	5.0	1.50	2.00					
6	BMV1+p	MT20	2.0	4.0							
7	BMWWW-t	MT20	3.0	6.0							
8	BMV1+p	MT20	2.0	4.0							

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

11-0-0

11-11-0

BEA	BEARINGS										
	FACTORED		MAXIMUM FACTORED			INPUT	REQRD				
	GROSS RI	GROSS REACTION			BRG	BRG					
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX				
8	671	0	709	-254	-295	5-8	5-8				
6	671	0	709	0	-295	5-8	5-8				

PROVIDE ANCHORAGE AT BEARING JOINT 8 FOR 295 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 6 FOR 295 LBS FACTORED UPLIFT

PROVIDE FOR 254 LBS FACTORED HORIZONTAL REACTION AT JOINT 8

	UNFACT	ORED	REACT	IONS
--	--------	------	-------	------

	1ST LCASE	MAX./	MIN. COMPON	NENT REACTION	DNS		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
8	469	341 / 0	0/0	0/0	94 / -292	127 / 0	0/0
6	469	341 / 0	0/0	0/0	94 / -292	127 / 0	0/0
HOR 8	IZONTAL RE	ACTIONS 0/0	0/0	0/0	181 / -181	0/0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 8, 6

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

<u>LOADING</u> TOTAL LOAD CASES: (11)

CHC	ORDS					WEI	BS		
MAX.	FACTORED	FACTO	RED				MAX. FACTO	RED	
MEMB.	FORCE	VERT. LO	AD LC	1 MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PL	.F) (	CSI (LC)	<b>UNBRAC</b>	0	(LBS)	CSI (LC)	
FR-TO		FROM	TO		LENGTH	FR-TO			
1- 2	0 / 29	-77.3	-77.3	0.10(1)	10.00	7-3	0 / 104	0.04 (11)	
2-3	-455 / 216	-77.3	-77.3	0.51 (7)	6.25	2-7	-73 / 393	0.08 (1)	
3- 4	-455 / 216	-77.3	-77.3	0.51 (8)	6.25	7- 4	-73 / 393	0.08 (1)	
4- 5	0 / 29	-77.3	-77.3	0.10(1)	10.00				
8- 2	-669 / 323	0.0	0.0	0.07(1)	7.81				
6- 4	-669 / 323	0.0	0.0	0.07(1)	7.81				
8- 7	-233 / 244	-17.5	-17.5	0.18 (11	6.25				
7-6	-10 / 21	-17.5	-17.5	0.18 (11	10.00				

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CPCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

### **DESIGN CRITERIA**

SPECIFIED LOADS: LL = DL = LL = DL = AD = CH. PSF 3.0 PSF

7.0 PSF TOTAL LOAD 33.3 PSF

### SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

TOTAL WEIGHT = 2 X 49 = 98 lb

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014 - CSA 086-09 TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.40") CALCULATED VERT. DEFL.(LL) = L/999 (0.01") ALLOWABLE DEFL.(TL)= L/360 (0.40") CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.51 (3-4:8) , BC=0.18 (7-8:11) , WB=0.08 (2-7:1) , SSI=0.16 (3-4:8)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (7) (INPUT = 0.90 ) JSI METAL= 0.19 (2) (INPUT = 1.00 )





READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

JOB NAME TW0317-048 TRUSS NAME QUANTITY PLY

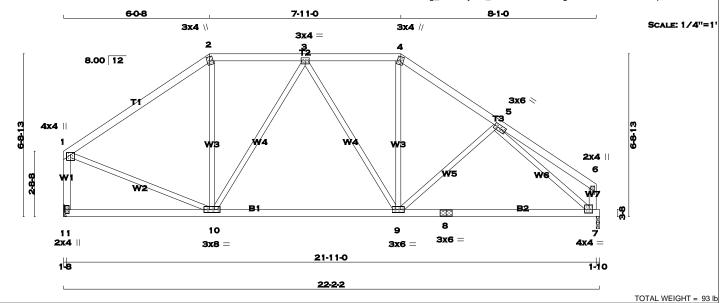
JOB PR-GREENPARK-LECCO RIDGE-BLOCK 327 TRUSS DESC

DRWG NO.

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**T77** Kott Lumber Uxbridge, Stouffville, ON, TW

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LUMBER				
N. L. G. A. R	ULES			
CHORDS	SIZE		LUMBER	DESCR.
1 - 2	2x4	DRY	No.2	SPF
2 - 4	2x4	DRY	No.2	SPF
4 - 6	2x4	DRY	No.2	SPF
11 - 1	2x4	DRY	No.2	SPF
7 - 6	2x4	DRY	No.2	SPF
11 - 8	2x4	DRY	No.2	SPF
8 - 7	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
FXCEPT				

DRY: SEASONED LUMBER.

	PLATES	table is ir	n inches)
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JT	TYPE	PLATES	W	LEN	Υ	Χ
1	TMVW+p	MT20	4.0	4.0	1.25	2.00
2	TTW+m	MT20	3.0	4.0	2.00	1.25
3	TMWW-t	MT20	3.0	4.0		
4	TTW+m	MT20	3.0	4.0	2.00	1.25
5	TMWW-t	MT20	3.0	6.0	1.50	2.00
6	TMV+p	MT20	2.0	4.0		
7	BMVW1-t	MT20	4.0	4.0	1.75	1.75
8	BS-t	MT20	3.0	6.0		
9	BMWWW-t	MT20	3.0	6.0		
10	BMWWW-t	MT20	3.0	8.0		
11	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

## DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

<u>EARINGS</u>										
FACTO	RED	MAXIMU	M FACT	ORED	INPUT	REQRD				
GROSS R	EACTION	GROSS	REACTIO	N	BRG	BRG				
VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX				
1045	0	1105	-300	-492	HANGER	BY OTHERS				
					MIN. SEA	AT SIZE: 1-8				
1045	0	1098	0	-505	1-10	1-10				
	FACTO GROSS RI VERT 1045	FACTORED GROSS REACTION VERT HORZ 1045 0	FACTORED MAXIMU GROSS REACTION GROSS VERT HORZ DOWN 1045 0 1105	FACTORED MAXIMUM FACT GROSS REACTION GROSS REACTION VERT HORZ DOWN HORZ 1045 0 1105 -300	FACTORED MAXIMUM FACTORED GROSS REACTION GROSS REACTION VERT HORZ DOWN HORZ UPLIFT 1045 0 1105 -300 -492	FACTORED MAXIMUM FACTORED INPUT GROSS REACTION GROSS REACTION BRG VERT HORZ DOWN HORZ UPLIFT IN-SX 1045 0 1105 -300 -492 HANGER MIN. SEA				

PROVIDE ANCHORAGE AT BEARING JOINT 11 FOR 492 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 7 FOR 505 LBS FACTORED UPLIFT UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES. SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 300 LBS FACTORED HORIZONTAL REACTION AT JOINT 11

## UNFACTORED REACTIONS

	1ST LCASE MAX./MIN. COMPONENT REACTIONS								
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
11	734	513/0	0/0	0/0	149 / -493	220 / 0	0/0		
7	734	513/0	0/0	0/0	131 / -503	220 / 0	0/0		
HOR 11	IZONTAL RE	ACTIONS 0/0	0/0	0/0	191 / -214	0/0	0 /0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 7

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.91 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (11)

	ORDS					W E	BS	
MAX	(. FACTORED	FACTO	RED				MAX. FACTO	DRED
MEMB.	FORCE	VERT. LO	AD LC	1 MAX	MAX.	MEMB	. FORCE	MAX
	(LBS)	(PL	.F) (	CSI (LC)	UNBRAC		(LBS)	CSI (LC)
FR-TO		FROM	TO		LENGTH	FR-TC	)	
1- 2	-926 / 466	-77.3	-77.3	0.57 (7)	5.93	10- 2	-53 / 149	0.04 (8)
2-3	-792 / 519	-77.3	-77.3	0.21 (7)	6.25	10-3	-362 / 285	0.40 (4)
3- 4	-899 / 584	-77.3	-77.3	0.22 (8)	6.25	3-9	-131 / 198	0.15 (3)
4- 5	-1087 / 614	-77.3	-77.3	0.30 (8)	5.91	9- 4	-154 / 347	0.11 (7)
5-6	-21 / 109	-77.3	-77.3	0.23 (8)	6.25	9- 5	-186 / 309	0.09 (4)
11- 1	-1063 / 521	0.0	0.0	0.14(1)	7.67	1-10	-275 / 800	0.20 (8)
7-6	-149 / 156	0.0	0.0	0.03 (8)	7.81	5- 7	-1325 / 576	0.63 (1)
44.40	004 / 070	47.5	47.5	0.47 (44				
11-10	-221 / 278	-17.5	-17.5	0.17 (11	) 6.25			
10-9	-335 / 963	-17.5	-17.5	0.34 (11	) 6.25			
9-8	-404 / 975	-17.5	-17.5	0.34 (11	) 6.25			
8- 7	-404 / 975	-17.5	-17.5	0.34 (11	) 6.25			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0}, FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CPC3, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEMS,INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

**RECEIVED** TOWN OF MILTON MAR 29, 2017 17-4978 **BUILDING DIVISION** 

# **DESIGN CRITERIA**

SPECIFIED LOADS:									
TOP	CH.	LL	=	23.3	PSF				
		DL	=	3.0	PSF				
BOT	CH.	LL	=	0.0	PSF				
		DL	=	7.0	PSF				
TOTA	L LO	AD	=	33.3	PSF				

### SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

### THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09

- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73") CALCULATED VERT. DEFL.(LL)= L/ 999 (0.03") ALLOWABLE DEFL.(TL)= L/360 (0.73") CALCULATED VERT. DEFL.(TL)= L/999 (0.13")

CSI: TC=0.57 (1-2:7), BC=0.34 (9-10:11), WB=0.63 (5-7:1) , SSI=0.16 (1-2:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

### NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (9) (INPUT = 0.90) JSI METAL= 0.37 (7) (INPUT = 1.00) JOB NAME TRUSS NAME

Kott Lumber Uxbridge, Stouffville, ON, TW

T78

TW0317-048

QUANTITY

PLY

POBPES GREENPARK-LECCO RIDGE-BLOCK 327 TRUSS DESC

DRWG NO.

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SCALE = 1:67.2

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7.9.8 12-0-8 3 8.00 12 3x5 / 3x4 < 2x4 ₩ W2 3x6 < 5 B1 W7 7 8 5x5 = 3x6 = 1988 1-10

LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR 3 2x4 DRY No.2 No.2 SPF SPF 2x4 2x4 DRY No 2 SPF 2x4 No.2 2x4 DRY No.2 SPF No.2 DRY ALL WEBS 2x3 DRY No.2 SPF EXCEPT

DRY: SEASONED LUMBER

PLATES (table is in inches)

JT	TYPE	PLATES	· W	LEN	Υ	X
1	TMV+p	MT20	2.0	4.0		
2	TMWW-t	MT20	3.0	5.0		
3	TTW+p	MT20	3.0	4.0	2.25	1.50
4	TMWW-t	MT20	3.0	4.0	1.50	1.50
5	TMVW-t	MT20	3.0	6.0		
6	BMV1+p	MT20	2.0	4.0		
7	BSWW-I	MT20	5.0	5.0	3.00	2.50
8	BMWWW-t	MT20	3.0	6.0		
9	BMVW1-t	MT20	4.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

19-11-10

REAL	EARINGS										
	FACTO	RED	MAXIMU	M FACT	ORED	INPUT	REQRD				
	GROSS R	EACTION	GROSS	REACTIO	N	BRG	BRG				
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX				
9	941	0	997	-433	-387	HANGER E	BY OTHERS				
						MIN. SEAT	SIZE: 1-8				
6	941	0	1004	0	-402	1-10	1-10				

PROVIDE ANCHORAGE AT BEARING JOINT 9 FOR 387 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 6 FOR 402 LBS FACTORED UPLIFT

PROVIDE FOR 433 LBS FACTORED HORIZONTAL REACTION AT JOINT 9

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS									
COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL			
660	462 / 0	0/0	0/0	142 / -404	198 / 0	0/0			
660	462 / 0	0/0	0/0	159 / -414	198 / 0	0/0			
HORIZONTAL REACTIONS									
	0/0	0/0	0/0	262 / -310	0/0	0 /0			
	COMBINED 660 660 ZONTAL REA	COMBINED SNOW 660 462 / 0 660 462 / 0 ZONTAL REACTIONS	COMBINED SNOW LIVE 660 462 / 0 0 / 0 660 462 / 0 0 / 0 ZONTAL REACTIONS	COMBINED         SNOW         LIVE         PERM.LIVE           660         462 / 0         0 / 0         0 / 0           660         462 / 0         0 / 0         0 / 0           ZONTAL REACTIONS         ZONTAL REACTIONS         20 / 0         0 / 0	COMBINED SNOW LIVE PERM.LIVE WIND 660 462/0 0/0 0/0 142/-404 660 462/0 0/0 0/0 159/-414  ZONTAL REACTIONS	COMBINED SNOW LIVE PERM.LIVE WIND DEAD 660 462 / 0 0 / 0 0 / 0 142 / -404 198 / 0 660 462 / 0 0 / 0 0 / 0 159 / -414 198 / 0 2ONTAL REACTIONS			

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 6

**BRACING**TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.63 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 3-8. DBS = 20-0-0 . CBF = 32 LBS. 1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 2-9. DBS = 16-0-0 . CBF = 90 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING TOTAL LOAD CASES: (11)

	ORDS	FACTO	RED			WE	B S MAX. FACTO	ORED
MEMB.	FORCE	VERT. LO			MAX.		FORCE	MAX
	(LBS)	(PL			UNBRAC		(LBS)	CSI (LC)
FR-TO		FROM	TO		LENGTH	FR-TO		
1- 2	-49 / 174	-77.3	-77.3	0.23(7)	6.25	2-8	-79 / 271	0.09 (5)
2-3	-682 / 469	-77.3	-77.3	0.26(7)	6.25	8-3	-298 / 430	0.16 (7)
3- 4	-696 / 424	-77.3	-77.3	0.55 (8)	6.25	8- 4	-561 / 499	0.69 (4)
4- 5	-1060 / 446	-77.3	-77.3	0.55 (8)	5.63	7- 4	-46 / 120	0.03 (11)
9- 1	-161 / 189	0.0	0.0	0.11 (7)	7.81	9- 2	-965 / 343	0.34 (4)
6- 5	-958 / 433	0.0	0.0	0.09(1)	7.81	7- 5	-255 / 913	0.20 (1)
9-8	-243 / 553	-17.5		0.26 (11)				
8- 7	-231 / 901			0.29 (11)				
7-6	-9 / 20	-17.5	-17.5	0.14 (11)	10.00			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF ( 9.0) PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPE APP AIN AND ATTOCATED ON BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

**RECEIVED** TOWN OF MILTON MAR 29, 2017 17-4978 **BUILDING DIVISION**  **DESIGN CRITERIA** 

SPECIFIED LOADS: LL = DL = LL = DL = AD = CH. 3.0 PSF 7.0 PSF

TOTAL LOAD 33.3 PSF SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

TOTAL WEIGHT = 4 X 89 = 357 lb

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09 - TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66") CALCULATED VERT. DEFL.(LL) = L/999 (0.03") ALLOWABLE DEFL.(TL)= L/360 (0.66") CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CSI: TC=0.55 (4-5:8), BC=0.29 (7-8:11), WB=0.69 (4-8:4) , SSI=0.20 (4-5:8)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION

(PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (2) (INPUT = 0.90 ) JSI METAL= 0.30 (5) (INPUT = 1.00 )



JOB NAME TRUSS NAME QUANTITY PLY TW0317-048 T79

Kott Lumber Uxbridge, Stouffville, ON, TW

JOB PESC TR-GREENPARK-LECCO RIDGE-BLOCK 327

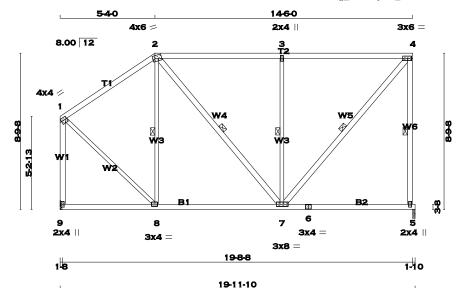
DRWG NO

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SCALE = 1:64.8

TOTAL WEIGHT = 105 lb

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LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR 2 2x4 DRY No.2 No.2 SPF SPF 2x4 DRY 2x4 DRY No 2 SPF 2x4 No.2 6 2x4 DRY No.2 SPF No.2 SPF ALL WERS 2x3 DRY No.2 SPF EXCEPT DRY SPF 2x4 No.2 SPF No.2

DRY: SEASONED LUMBER

PLATES	(table is in inches)	

JT	TYPE	PLATES	W	LEN	Υ	Χ
1	TMVW-t	MT20	4.0	4.0	1.75	Edge
2	TTWW-m	MT20	4.0	6.0	1.75	1.50
3	TMW+w	MT20	2.0	4.0		
4	TMVW-t	MT20	3.0	6.0	1.50	2.75
5	BMV1+p	MT20	2.0	4.0		
6	BS-t	MT20	3.0	4.0		
7	BMWWW-t	MT20	3.0	8.0	1.50	2.50
8	BMWW-t	MT20	3.0	4.0	1.50	1.75
9	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

# DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS

	1400						
	FACTOR	RED	MAXIMUM FACTORED			INPUT	REQRD
	GROSS RE	ACTION	GROSS F	REACTIO	BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
5	941	0	1007	0	-531	1-10	1-10
9	941	0	989	465	-461	HANGER E	BY OTHERS
						MIN. SEAT	SIZE: 1-8

PROVIDE ANCHORAGE AT BEARING JOINT 5 FOR 531 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 9 FOR 461 LBS FACTORED UPLIFT

ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES. SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 465 LBS FACTORED HORIZONTAL REACTION AT JOINT 9

## UNFACTORED REACTIONS

	1ST LCASE	MAX./	MIN. COMPO	NENT REACTION	ONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
5	660	462 / 0	0/0	0/0	165 / -507	198 / 0	0/0		
9	660	462 / 0	0/0	0/0	121 / -457	198 / 0	0/0		
HOR	HORIZONTAL REACTIONS								
9		0/0	0/0	0/0	332 / -280	0/0	0 /0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 5

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.03 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 4-5. DBS = 16-0-0 . CBF = 89 LBS 1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 2-8, 2-7, 3-7, 4-7. DBS = 20-0-0 . CBF = 87 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING TOTAL LOAD CASES: (11)

СНС	RDS					WE	BS	
MAX.	FACTORED	FACTO	RED				MAX. FACTO	DRED
MEMB.	FORCE	VERT. LO	AD LC	1 MAX	MAX.	MEMB.	FORCE	MAX
	(LBS)	(PL	.F) (	CSI (LC)	UNBRAC		(LBS)	CSI (LC)
FR-TO		FROM	TO		LENGTH	FR-TO		
1- 2	-590 / 396	-77.3	-77.3	0.45 (7)	6.25	8- 2	-296 / 189	0.14 (4)
2-3	-632 / 505	-77.3	-77.3	0.73 (1)	6.03	2- 7	-154 / 253	0.10 (8)
3- 4	-632 / 506	-77.3	-77.3	0.73 (1)	6.03	7- 3	-734 / 565	0.35 (3)
5- 4	-955 / 567	0.0		0.50 (7)			-518 / 961	0.34 (7)
9- 1	-953 / 487	0.0	0.0	0.46 (1)	7.81	1- 8	-160 / 615	0.16 (8)
9-8	-369 / 347			0.14 (11				
8- 7	-345 / 500	-17.5	-17.5	0.26 (11	) 6.25			
7-6	-65 / 167			0.23 (11				
6- 5	-65 / 167	-17.5	-17.5	0.23 (11	) 6.25			

RENCE VELOCITY PRESSURE OF { 9.0} PSF AT FE GRADE AND USING EXTERNAL PEAK
WIND FORCE RESISTING SYSTEM, INTERNAL
TEGORY 2, BUILDING MAY BE LOCATED ON
TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

## **DESIGN CRITERIA**

SPECIFIED LOADS:								
TOP	CH.	LL	=	23.3	PSF			
		DL	=	3.0	PSF			
BOT	CH.	LL	=	0.0	PSF			
		DL	=	7.0	PSF			
TOTA	L LO	AD	=	33.3	PSF			

### SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

### THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09

- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66°) CALCULATED VERT. DEFL.(LL)= L/ 999 (0.02°) ALLOWABLE DEFL.(TL)= L/360 (0.66°) CALCULATED VERT. DEFL.(TL)= L/999 (0.09°)

CSI: TC=0.73 (3-4:1), BC=0.26 (7-8:11), WB=0.35 (3-7:3), SSI=0.28 (3-4:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

### NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (8) (INPUT = 0.90) JSI METAL= 0.26 (1) (INPUT = 1.00)



JOB NAME TRUSS NAME QUANTITY TW0317-048 T80

POBPES GREENPARK-LECCO RIDGE-BLOCK 327 TRUSS DESC.

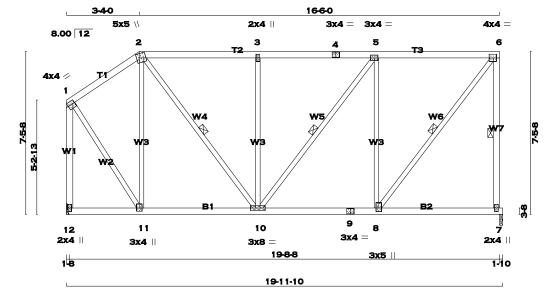
DRWG NO

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SCALE = 1:52.7

TOTAL WEIGHT = 102 lb

Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:21:10 2017 Page 1 Kott Lumber Uxbridge, Stouffville, ON, TW ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-apdNaEybLkNSEqWr\_3mX0o6bZ?vMwXkQP9VaB8zcJJN



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR 2 2x4 DRY No.2 No.2 SPF SPF 2x4 4 -7 -12-6 2x4 DRY No 2 SPF 6 2x4 No.2 2x4 DRY No.2 SPF SPF 12 -No.2 DRY 2x4 No.2 ALL WEBS EXCEPT 2x3 No.2

DRY: SEASONED LUMBER.

PLATES	table is	s in i	nches)

	TIEG (tubic	15 III IIICIIC	2,			
JT	TYPE	PLATES	W	LEN	Υ	Χ
1	TMVW-t	MT20	4.0	4.0	1.75	Edge
2	TTWW+m	MT20	5.0	5.0	2.25	1.50
3	TMW+w	MT20	2.0	4.0		
4	TS-t	MT20	3.0	4.0		
5	TMWW-t	MT20	3.0	4.0		
6	TMVW-t	MT20	4.0	4.0	2.00	1.75
7	BMV1+p	MT20	2.0	4.0		
8	BMWW+t	MT20	3.0	5.0	2.00	1.50
9	BS-t	MT20	3.0	4.0		
10	BMWWW-t	MT20	3.0	8.0		
11	BMWW+t	MT20	3.0	4.0	1.75	1.50
12	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

# DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS

DEA	KINGS						
	FACTO	RED	MAXIMUM FACTORED			INPUT	REQRD
	GROSS R	EACTION	GROSS	REACTIO	N	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
7	941	0	999	0	-516	1-10	1-10
12	941	0	984	390	-476	HANGER E	BY OTHERS
						MIN. SEAT	SIZE: 1-8

PROVIDE ANCHORAGE AT BEARING JOINT 7 FOR 516 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 12 FOR 476 LBS FACTORED UPLIFT

ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES. SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 390 LBS FACTORED HORIZONTAL REACTION AT JOINT 12

## UNFACTORED REACTIONS

	1ST LCASE	MAX./	MIN. COMPO	NENT REACTION	ONS		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
7	660	462 / 0	0/0	0/0	145 / -496	198 / 0	0/0
12	660	462 / 0	0/0	0/0	109 / -468	198 / 0	0/0
HOR 12	IZONTAL RE	ACTIONS 0/0	0/0	0/0	279 / -247	0/0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 7

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 6-7. DBS = 16-0-0 . CBF = 90 LBS. 1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 2-10, 5-10, 6-8. DBS = 20-0-0 . CBF = 58 LBS

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING TOTAL LOAD CASES: (11)

СНО	ORDS					WE	BS	
MAX.	FACTORED	FACTOR	RED				MAX. FACTO	ORED
MEMB.	FORCE	VERT. LO	AD LC	1 MAX	MAX.	MEMB.	FORCE	MAX
	(LBS)	(PL	.F) (	CSI (LC)	UNBRA(	0	(LBS)	CSI (LC)
FR-TO		FROM	TO		LENGTH	FR-TO		
1- 2	-512 / 363	-77.3	-77.3	0.20 (7)	6.25	11- 2	-457 / 245	0.45 (1)
2- 3	-736 / 529	-77.3	-77.3	0.31(1)	6.25	2-10	-287 / 523	0.15 (8)
3- 4	-736 / 529			0.34(1)		10- 3	-478 / 368	0.47 (3)
4- 5	-736 / 529			0.34(1)			-140 / 206	0.08 (6)
5- 6	-611 / 457	-77.3		0.34 (1)		8- 5		0.65 (3)
7-6	-959 / 544	0.0		0.37 (7)		8- 6	-511 / 984	0.28 (7)
12- 1	-965 / 491	0.0	0.0	0.47 (1)	7.81	1-11	-226 / 666	0.15 (1)
12-11	-294 / 301			0.08 (11				
11-10	-312 / 425	-17.5		0.12 (11				
10-9	-266 / 606	-17.5	-17.5	0.17 (11	) 6.25			
9- 8	<u>-2</u> 66/606	-17.5	-17.5	0.17 (11	) 6.25			
8- 7	-5° / 14		_	(	) 6.25			
							-	
WIND	AD PP	R		M R F	SENCE V	'ELOCIT	Y PRESSURE	OF ( 9 0) PSF

WIND AD PPORT REPORTED OM RESERVE VELOCITY M (40-0°), --IN-SX-RESERVE HEIGHT ABOVE GRADE AND USING COEFFICIENTS, CPCg, BASED ON THE (MAIN WIND FORCE RESIS

WIN READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

EXTERNE GENVED INCTOWNEMONTEMPLATION
MAY BE LOCATED ON EAST {**M/0A/RT-12/9**\$*X***2/0/1/**RY

17-4978 **BUILDING DIVISION** 

## **DESIGN CRITERIA**

SPECIFIED LOADS:									
TOP	CH.	LL	=	23.3	PSF				
		DL	=	3.0	PSF				
BOT	CH.	LL	=	0.0	PSF				
		DL	=	7.0	PSF				
TOTA	J IO	AD	=	33.3	PSF				

### SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

# THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09 - TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66°) CALCULATED VERT. DEFL.(LL)= L/ 999 (0.03°) ALLOWABLE DEFL.(TL)= L/360 (0.66°) CALCULATED VERT. DEFL.(TL)= L/999 (0.05°)

CSI: TC=0.47 (1-12:1), BC=0.17 (8-10:11), WB=0.65 (5-8:3), SSI=0.20 (5-6:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

### NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (8) (INPUT = 0.90) JSI METAL= 0.26 (8) (INPUT = 1.00)



JOB NAME TRUSS NAME

Kott Lumber Uxbridge, Stouffville, ON, TW

T81

TW0317-048

QUANTITY

3

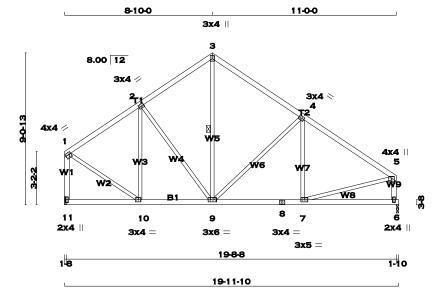
JOB PESC TR-GREENPARK-LECCO RIDGE-BLOCK 327 TRUSS DESC.

DRWG NO.

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SCALE = 1:68.8

Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:21:10 2017 Page 1 ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-apdNaEybLkNSEqWr\_3mX0o6bW?u\_waBQP9VaB8zcJJN



LUMBER N. L. G. A. CHORDS LUMBER DESCR SIZE 3 2x4 DRY No.2 No.2 SPF SPF 2x4 11 -2x4 DRY No 2 SPF 6 -11-2x4 No.2 8 2x4 DRY No.2 SPF 6 No.2 DRY ALL WERS 2x3 DRY No.2 SPF EXCEPT

DRY: SEASONED LUMBER

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Υ	X
1	TMVW-t	MT20	4.0	4.0	1.50	1.00
2	TMWW-t	MT20	3.0	4.0	1.50	1.50
3	TTW+p	MT20	3.0	4.0	2.25	1.50
4	TMWW-t	MT20	3.0	4.0	1.50	1.50
5	TMVW+p	MT20	4.0	4.0	1.25	2.00
6	BMV1+p	MT20	2.0	4.0		
7	BMWW-t	MT20	3.0	5.0		
8	BS-t	MT20	3.0	4.0		
9	BMWWW-t	MT20	3.0	6.0		
10	BMWW-t	MT20	3.0	4.0	1.50	1.75
11	BMV1+p	MT20	2.0	4.0		
	•					

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY **BUILDING DESIGNER** 

BEA	RINGS						
	FACTO	RED	MAXIMU	IM FACT	INPUT	REQRD	
	GROSS R	EACTION	GROSS	REACTIO	N	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
11	941	0	989	-399	-372	HANGER	BY OTHERS
						MIN. SEA	T SIZE: 1-8
6	941	0	1001	0	-395	1-10	1-10

PROVIDE ANCHORAGE AT BEARING JOINT 11 FOR 372 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 6 FOR 395 LBS FACTORED UPLIFT UPLIFT

PROVIDE FOR 399 LBS FACTORED HORIZONTAL REACTION AT JOINT 11

UNFACTORED REACTIONS

	1ST LCASE	MAX.	MIN. COMPON	ENT REACTION	ONS			
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
11	660	462 / 0	0/0	0/0	121 / -393	198 / 0	0/0	
6	660	462 / 0	0/0	0/0	150 / -410	198 / 0	0/0	
HORIZONTAL REACTIONS								
11		0/0	0/0	0/0	261 / -285	0/0	0 /0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 6

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.89 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF 3-9. DBS = 20-0-0 . CBF = 35 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING TOTAL LOAD CASES: (11)

СНО	DRDS					WEBS			
MAX.	FACTORED	FACTO	RED				MAX. FACTO	RED	
MEMB.	FORCE	VERT. LO	AD LC	1 MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PL	.F) (	CSI (LC)	<b>UNBRAC</b>		(LBS)	CSI (LC)	
FR-TO		FROM	TO		LENGTH	FR-TO		, ,	
1- 2	-740 / 364	-77.3	-77.3	0.31 (7)	6.25	10- 2	-316 / 187	0.19 (4)	
2-3	-721 / 476	-77.3	-77.3	0.32(7)	6.25	2-9	-177 / 293	0.18 (3)	
3- 4	-727 / 452	-77.3	-77.3	0.47 (8)	6.25	9-3	-319 / 469	0.16 (7)	
4- 5	-990 / 420	-77.3	-77.3	0.47 (8)	5.89	9- 4	-456 / 425	0.49 (4)	
11-1	-956 / 394	0.0	0.0	0.16(1)	7.81	7- 4	-104 / 143	0.04(3)	
6- 5	-960 / 422	0.0	0.0	0.10(1)	7.81	1-10	-233 / 750	0.16(1)	
						7- 5	-251 / 869	0.19(1)	
11-10	-309 / 373	-17.5	-17.5	0.08 (5)	6.25				
10-9	-256 / 689	-17.5	-17.5	0.14(1)	6.25				
9-8	-216 / 842	-17.5	-17.5	0.20(1)	6.25				
8- 7	-216 / 842	-17.5	-17.5	0.20(1)	6.25				
7-6	-13 / 28	-17.5	-17.5	0.13 (11	) 6.25				

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK
COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL
WIND BEST UR STANSON OF THE WAIN WIND FORCE RESISTING SYSTEM).INTERNAL
WIND BEST UR STANSON OF THE WAIN WIND FORCE RESISTING SYSTEM).INTERNAL
WIND BEST UR STANSON OF THE WAIN WIND FORCE RESISTING SYSTEM OF THE WAIN WIND FORCE WAIN WIN TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

RECEIVED TOWN OF MILTON MAR 29, 2017 17-4978 **BUILDING DIVISION**  **DESIGN CRITERIA** 

SPECIFIED LOADS: LL = DL = LL = DL = AD = CH. 3.0 PSF 7.0 PSF TOTAL LOAD 33.3

24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

PSF

TOTAL WEIGHT = 3 X 90 = 271

THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012, BCBC 2012, ABC 2014 - CSA 086-09 - TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66") CALCULATED VERT. DEFL.(LL) = L/999 (0.02") ALLOWABLE DEFL.(TL)= L/360 (0.66") CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.47 (3-4:8), BC=0.20 (7-9:1), WB=0.49 (4-9:4) , SSI=0.18 (4-5:8)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (10) (INPUT = 0.90 ) JSI METAL= 0.31 (1) (INPUT = 1.00 )



JOB NAME TW0317-048

Kott Lumber Uxbridge, Stouffville, ON, TW

TRUSS NAME QUANTITY 2 T82

PLY

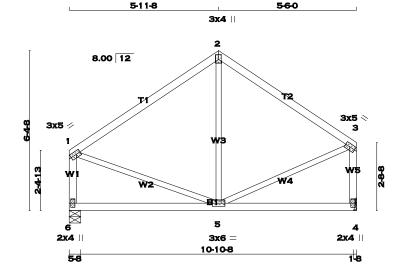
JOB PESC TR-GREENPARK-LECCO RIDGE-BLOCK 327 TRUSS DESC.

DRWG NO

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SCALE = 1:45.9

Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:21:10 2017 Page 1 ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-apdNaEybLkNSEgWr\_3mX0o6bv?vTwgtQP9VaB8zcJJN



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR 2 2x4 2x4 DRY No.2 No.2 SPF SPF DRY 2x4 DRY No 2 SPF 6 4 6 2x4 No.2 2x4 DRY No.2 SPF ALL WEBS DRY No.2 SPF 2x3 EXCEPT

DRY: SEASONED LUMBER

PLATES (table is in inches)
JT TYPE PLATES
1 TMVW-t MT20 W LEN Y 1.50 2.00 3.0 MT20

5.0 4.0 5.0 4.0 3.0 3.0 2.0 TTW+p TMVW-t 2.25 1.50 1.50 2.00 BMV1+p MT20 BMWWW-t MT20 3.0 6.0 BMV1+p MT20 4.0

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS

11-5-8

DEA	KINGS						
	FACTORED		MAXIMU	M FACT	INPUT	REQRD	
	GROSS RE	EACTION	GROSS I	REACTIO	N	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
6	543	0	573	282	-215	5-8	5-8
4	543	0	575	0	-214	HANGER E	BY OTHERS
						MIN. SEAT	SIZE: 1-8

PROVIDE ANCHORAGE AT BEARING JOINT 6 FOR 215 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT 4 FOR 214 LBS FACTORED UPLIFT

PROVIDE FOR 282 LBS FACTORED HORIZONTAL REACTION AT JOINT 6

UNFACTORED REACTIONS

	1ST LCASE	MAX./	MIN. COMPO	ONS			
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
6	381	267 / 0	0/0	0/0	74 / -227	115/0	0/0
4	381	267 / 0	0/0	0/0	80 / -227	115 / 0	0/0
HORIZONTAL REAG		ACTIONS 0/0	0/0	0/0	201 / -196	0/0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 6

**BRACING**TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (11)

CHC	RDS					WE	BS		
MAX.	FACTORED	FACTOR	ED				MAX. FACTO	RED	
MEMB.	FORCE	VERT. LOA	AD LC1	1 MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PLF	=) (	CSI (LC)	<b>UNBRAC</b>	)	(LBS)	CSI (LC)	
FR-TO		FROM 1	ΓO		LENGTH	FR-TO	, ,		
1-2	-359 / 201	-77.3	-77.3	0.51 (7)	6.25	5- 2	-89 / 92	0.06(3)	
2-3	-359 / 210	-77.3	-77.3	0.44 (8)	6.25	1- 5	-79 / 318	0.06(1)	
6- 1	-533 / 244	0.0	0.0	0.06(1)	7.81	5-3	-84 / 327	0.07(1)	
4- 3	-540 / 239	0.0	0.0	0.07 (1)	7.81				
6- 5	-241 / 256	-17.5	-17.5	0.17 (11)	6.25				
5- 4	-22 / 47	-17.5	-17.5	0.17 (11)	6.25				

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF ( 9.0) PSE AT WIND LOAD APPLIED IS DERIVED FROM REPERSING VELOCITY FRESSURE OF (3.9), 25 AT (40-0.0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.

### **DESIGN CRITERIA**

SPECIFIED LOADS:

LL = DL = LL = DL = AD = CH. PSF 3.0 PSF PSF 7.0 PSF TOTAL LOAD 33.3 PSF

### SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

TOTAL WEIGHT = 2 X 48 = 96 lb

THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012, BCBC 2012, ABC 2014 - CSA 086-09

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F.

TPIC 2011

RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.38") CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.51 (1-2:7), BC=0.17 (4-5:11), WB=0.07 (3-5:1) , SSI=0.16 (1-2:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.53 (5) (INPUT = 0.90 ) JSI METAL= 0.15 (1) (INPUT = 1.00 )





READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

JOB PR-GREENPARK-LECCO RIDGE-BLOCK 327 JOB NAME TRUSS NAME QUANTITY DRWG NO. TRUSS DESC TW0317-048 T83 Kott Lumber Uxbridge, Stouffville, ON, TW

Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:21:11 2017 Page 1 ID:4BORDhR74Ei?Dog\_xdfUkJyKZi\_-20AloazD61VJs\_51YnHmY0frZPHef8QaepE8jazcJJM

01-0-0

1-11-4 2-1-8 4x5 = 3x4 // 8.00 12 2 3x4 / 3x4 < 14-13 We W5 B1 7 6 3x4 = 3x6 = 2x4 || 2x4 || 640 5-8 6-11-0

TOTAL WEIGHT = 31 lb

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SCALE = 1:26.1

TW0317-048

LUMBER				
N. L. G. A. R	ULES			
CHORDS	SIZE		LUMBER	DESCR.
1 - 2	2x4	DRY	No.2	SPF
2 - 3	2x4	DRY	No.2	SPF
3 - 4	2x4	DRY	No.2	SPF
8 - 1	2x4	DRY	No.2	SPF
5 - 4	2x4	DRY	No.2	SPF
8 - 5	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER

## PLATES (table is in inches)

JT	TYPE	PLATES		LEN	Υ	Χ
1	TMVW-t	MT20	3.0	4.0	1.50	1.00
2	TTWW-m	MT20	4.0	5.0	1.75	1.50
3	TTW+m	MT20	3.0	4.0		
4	TMVW-t	MT20	3.0	4.0	1.50	1.00
5	BMV1+p	MT20	2.0	4.0		
6	BMWWW-t	MT20	3.0	6.0		
7	BMWW-t	MT20	3.0	4.0		
8	BMV1+p	MT20	2.0	4.0		
	•					

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

DEAL	VIII G						
	FACTOR	RED	MAXIMUI	M FACT	INPUT	REQRD	
	<b>GROSS RE</b>	ACTION	GROSS REACTION			BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	<b>UPLIFT</b>	IN-SX	IN-SX
8	328	0	341	-150	-146	HANGER E	BY OTHERS
						MIN. SEAT	SIZE: 5-8
5	328	0	342	0	-151	HANGER E	BY OTHERS
						MIN. SEAT	SIZE: 1-8

PROVIDE FOR 150 LBS FACTORED HORIZONTAL REACTION AT JOINT 8

## UNFACTORED REACTIONS

	1ST LCASE	MAX./N	MAX./MIN. COMPONENT REACTIONS								
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL				
8	230	161 / 0	0/0	0/0	33 / -149	69 / 0	0/0				
5	230	161 / 0	0/0	0/0	36 / -152	69 / 0	0/0				
HOF	HORIZONTAL REACTIONS										
8		0/0	0/0	0/0	97 / -107	0/0	0 /0				

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (11)

CHC	DRDS				WEBS				
MAX.	FACTORED	FACTO	RED				MAX. FACTO	DRED	
MEMB.	FORCE	VERT. LO	AD LC	1 MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PL	.F) (	CSI (LC)	UNBRAC	;	(LBS)	CSI (LC)	
FR-TO		FROM	TO		LENGTH	FR-TO			
1- 2	-207 / 137	-77.3	-77.3	0.07(7)	6.25	7- 2	-102 / 65	0.02(1)	
2-3	-201 / 166	-77.3	-77.3	0.06(8)	6.25	2-6	-53 / 71	0.01 (4)	
3- 4	-243 / 128	-77.3	-77.3	0.13 (8)	6.25	6-3	-76 / 86	0.02 (8)	
8- 1	-325 / 157	0.0	0.0	0.04(7)	7.81	1- 7	-69 / 215	0.05(1)	
5- 4	-319 / 165	0.0	0.0	0.03 (1)	7.81	6- 4	-48 / 215	0.04 (1)	
8- 7	-103 / 135	-17.5	-17.5	0.03 (5)	6.25				
7-6	-83 / 177	-17.5	-17.5	0.04 (11)	6.25				
6- 5	-10 / 21	-17.5	-17.5	0.03 (11)	10.00				

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF ( 9.0) PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.

### **DESIGN CRITERIA**

SPEC	IFIED	LOAI	DS:		
TOP	CH.	LL	=	23.3	PS
		DL	=	3.0	PS
BOT	CH.	LL	=	0.0	PS
		DL	=	7.0	PS

### SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014 - CSA 086-09

- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.23")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.23")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.00")

CSI: TC=0.13 (3-4:8), BC=0.04 (6-7:11), WB=0.05 (1-7:1), SSI=0.08 (3-4:8)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

### NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.65 (4) (INPUT = 0.90 ) JSI METAL= 0.11 (4) (INPUT = 1.00 )





READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

# BEARING ANCHORAGE BY TOE-NAILS FOR LATERAL CAPACITY

B37579H1

NAIL TYPE	LENGTH	DIAMETER	NAIL LATERAL CAPACITY (LE	
NAILTTPE	(JN)	(IN)	S-P-F	D. FIR
COMMON	3.00	0.144	132	147
WIRE	3.25	0.144	132	147
WIRE	3.50	0.160	159	177
COMMON	3.00	0.122	97	108
SPIRAL	3.25	0.122	97	108
SFIRAL	3.50	0.152	145	162

### NOTES:

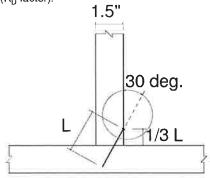
- 1. Rafter and ceiling members may be anchored to top and bottom chords of girder truss by toe-nailing rafter and ceiling members to girder chords provided the reaction does not exceed the lateral capacities in the table. Hangers (specified by others) are required for reactions higher than the maximum toe-nail capacity. Reactions are based on factored loads.
- 2. Toe nail capacities shown in the table are for one toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J₄ in CSA O86-09, section 10.9.4.1.
- 3. For 9-3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
- 4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in tables below.
- 5. Nail values in table are based on the following relative lumber densities: G = 0.42 (SPF), G = 0.49 (D. Fir).
- 6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See next page for nailing on bearing plate).
- 7. For loads due to **wind** the nail lateral capacity in this table may be multiplied by 1.15 (K<sub>D</sub> factor).

8. Lumber must be dry ( < 19% moisture content ) at the time of nail installation.

9. Nail values in this table comply with CSA O86-09, section 10.9.4

10. This design is not valid after April 30, 2017.





TOE	NAIL	INST	ALL	ATION

Nail type	Common wire	Common spiral	Common wire	Common spiral
Nail dia. (in)	0.160	0.152	0.144	0.122
	( 3.5" nail )		( 3" and 3.25" nail )	
LUMBER SIZE	MAXIMUM NUMBER OF TOE-NAILS			
2X4 SPF	2	2	3	3
2X4 D. Fir	2	2	2	2

2X6	SPF	4	4	4	5
2X6	D. Fir	3	3	3	4



MiTek Canada Inc 100 Industrial Rd Bradford, Ontario L3Z 3G7

**RECEIVED** TOWN OF MILTON MAR 29, 2017 17-4978 **BUILDING DIVISION** 

I R R U D

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April 24, 2015

# BEARING ANCHORAGE BY TOE-NAILS FOR WIND LOADING

B37579H2

NAIL TYPE	LENGTH	DIAMETER	NAIL WITHDRAWAL CAPACITY (LB)		
NAIL ITPE	(IN)	(IN)	S-P-F	D. FIR	]ו
COMMON	3.00	0.144	30	42	][
WIRE	3.25	0.144	32	45	]k
AAIUE	3.50	0.160	38	52	]i
COMMON	3.00	0.122	26	36	]
SPIRAL	3.25	0.122	28	40	]
SFINAL	3.50	0.152	36	50	

**Note:** If using truss with D. Fir lumber and S-P-F bearing plate, use values in table for S-P-F.

### NOTES:

- 1. Truss chord, rafter, or ceiling members may be anchored to bearing plate by toe-nails, provided that the actual factored uplift force due to **wind** or **earthquake** load does not exceed the withdrawal capacities in the table. Hangers (specified by others) are required for uplift forces that are higher than the maximum toe-nail withdrawal capacity.
- 2. Toe nail capacities shown in the table are for one toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J<sub>A</sub> in CSA O86-09, section 10.9.5.2.
- 3. For 9-3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
- **4.** Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in table above.
- 5. Nail values in table are based on the following relative lumber densities: G = 0.42(SPF), G = 0.49(D. Fir).
- **6.** Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See drawing on detail B37579H1).
- 7. Lumber must be dry ( < 19% moisture content ) at the time of nail installation.
- 8. Nail values in this table comply with CSA O86-09, section 10.9.5
- 9. This design is not valid after April 30, 2017

