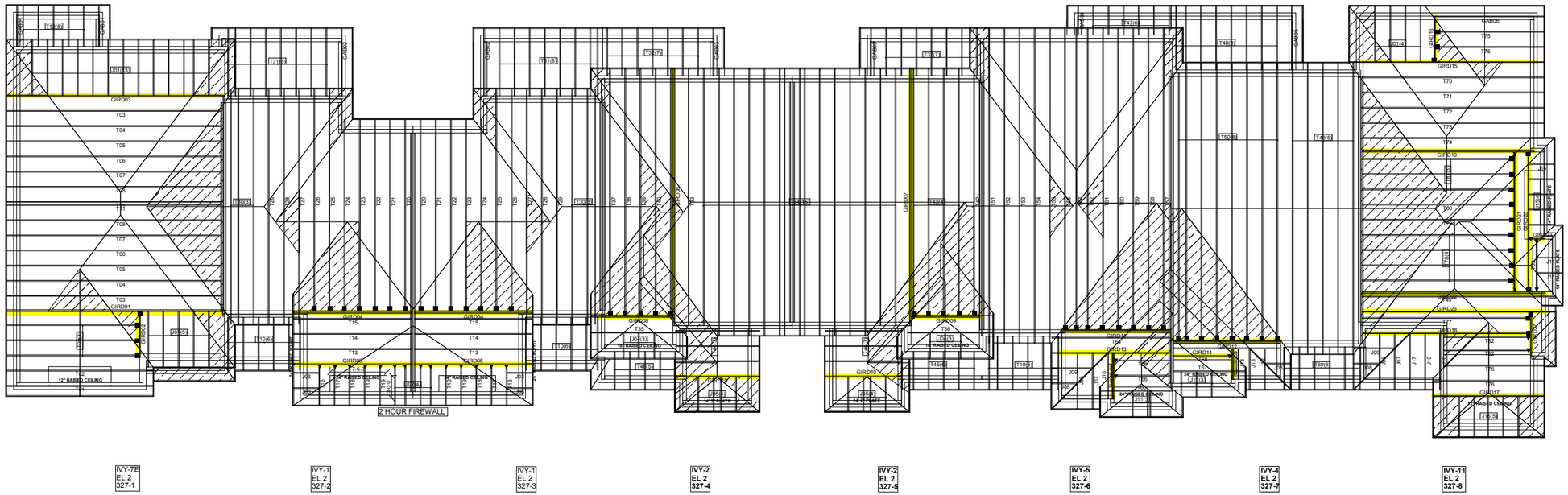
TOWN OF MILTON
PLANNING AND DEVELOPMENT
BUILDING PERMIT: 17-4978

BUILDING: REVIEWED
SCOTT SHERRIFFS APR 19, 2017
PLANS EXAMINER DATE

Neither the issuance of a permit nor carrying out of inspections by the Town of Milton relives the owner from full responsibility for compliance with the provisions of the Ontario Building Code Act and the Ontario Building Code, both as amended, as well as other applicable statutes and regulations of the Province on Ontario, By-laws of the Region of Halton and Town of Milton

RECEIVED
TOWN OF MILTON
MAR 29, 2017
17-4978
BUILDING DIVISION

TRUSSES BY KOTT



TOP OF ALL PORCH BEAMS
AT 8' 7 1/4 A.F.F.

UNDERSIDE OF SOFFIT
FOR PORCHES AT
8' 6" A.F.F.

GARAGE WALL PLATES
AT 9' 1 2/16" A.F.F.

PARTY WALLS TO BE
FRAMED ON SITE A.P.P.

HANGER LEGEND:

▼ LUS24 ■ LJS26DS
● HGUS26 ╳ HGUS26-2

CONVENTIONAL FRAMING BY OTHERS

SIZE AND LOCATION OF CONVENTIONAL FRAMING IS APPROXIMATE. ALL AREAS MAY NOT BE SHOWN. REFER TO ARCHITECTURAL PLANS FOR DETAILS.

ALL CONVENTIONAL FRAMING TO CONFORM WITH PART 9 OF THE O.B.C
ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4 S.P.F. @ 24"O/C WITH A 2x4 VERTICAL POST TO THE TRUSS UNDERNEATH EACH CROSS POINT. VERTICAL POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END POINT AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

Model: **BLOCK 327**
Customer: GREENPARK
Project: LECCO RIDGE
Location: MILTON
Date: 3/9/2017 Drawn by: BB

ENGINEERING NOTE PAGE (ENP-1)
PLEASE READ PRIOR TO INSTALLATION

RESPONSIBILITIES

THIS DESIGN IS FOR AN INDIVIDUAL BUILDING COMPONENT AND HAS BEEN BASED ON INFORMATION PROVIDED BY THE DESIGN OFFICE OF KOTT LUMBER. THE UNDERSIGNED ENGINEER DISCLAIMS ANY RESPONSIBILITY FOR DAMAGES AS A RESULT OF FAULTY OR INCORRECT INFORMATION, SPECIFICATION AND/OR DESIGNS FURNISHED TO THE ENGINEER. THE UNDERSIGNED ENGINEER IS ONLY RESPONSIBLE FOR THE STRUCTURAL INTEGRITY OF THIS BUILDING COMPONENT FOR THE CONDITIONS AND LOADS SHOWN ON THIS DRAWING. THE STRUCTURAL INTEGRITY OF THE BUILDING AND THE VERIFICATION OF THE DIMENSIONS AND THE DESIGN LOADS USED ARE THE RESPONSIBILITY OF THE BUILDING DESIGNER.

TRUSSES ARE DESIGNED IN CONFORMANCE WITH THE RELEVANT SECTIONS OF THE NATIONAL BUILDING CODE OF CANADA OR THE CANADIAN CODE FOR FARM BUILDINGS, WHICHEVER APPLIES TO THE BUILDING TYPE INDICATED ON THE DRAWING

IT IS THE RESPONSIBILITY OF KOTT LUMBER TO ENSURE THAT TRUSSES ARE MANUFACTURED IN CONFORMANCE WITH THESE DESIGNS AND WITH THE SPECIFICATIONS OUTLINED BELOW. THE UNDERSIGNED ENGINEER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

USE AND OCCUPANCY

- The building is of the type indicated on the drawing

LOADING

- The truss loading intensity and distribution as well as load transfer mechanism is that indicated on the drawing
- No buildings, trees, parapets or other projections higher than the roof for which the trusses are used are located within a distance less than ten (10) times the difference in height, or five metres (16 ft) whichever is greater, unless the drawing indicates that the snow drifting has been taken into account

HANDLING, INSTALLATION AND BRACING

- The trusses must be handled and installed by a qualified professional as per the supplied document titled *Information for Truss Installers* and the BCSI-B1 and BCSI-B3 Summary Sheets
- The compression chords are laterally braced by continuous rigid diaphragm sheathing or as specified on the drawing
- Temporary and permanent bracing must be installed as indicated on the truss drawing and according to the BCSI-B1 and BCSI-B3 Summary Sheets. Bracing for the lateral stability of the truss is to be provided by the building designer
- **It is recommended that a Professional Engineer's advice be obtained for the bracing of trusses spanning more than 12.37m (40'-7")**

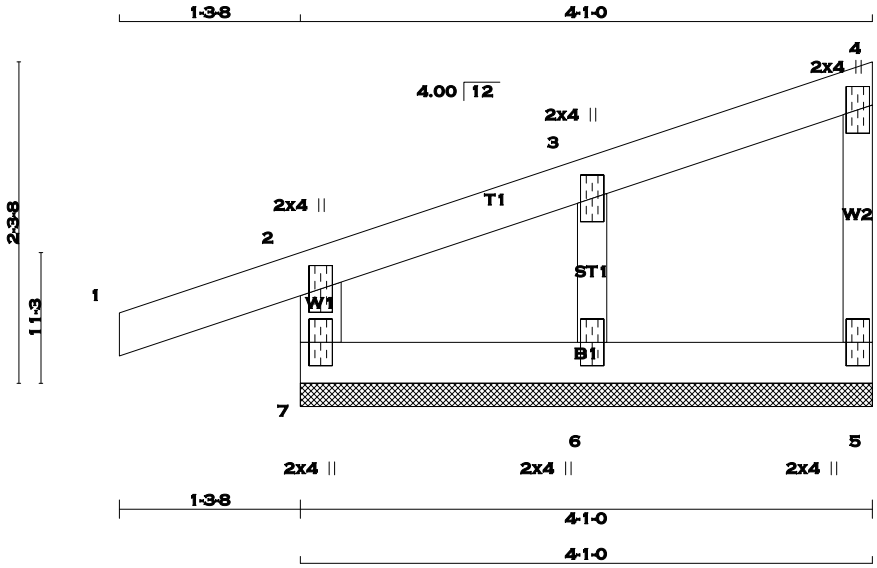
SUPPORTS

- The trusses are to be supported at the bearing points indicated and anchored to the supports where considered necessary by the designer of the overall structure
- Bearing sizes shown are the minimum required to prevent crushing of the truss members and do not necessarily take into account stability of the overall building structure
- Elevation of bearings must be carefully checked and shimmed to alignment for solid bearings
- Adequate wood truss bearing is the responsibility of the building designer.

DIMENSIONS

- Geometry of the truss and dimensions indicated on the drawing are identical to those of the installed truss.

01/29/2013



SCALE = 1:16.4

TOTAL WEIGHT = 2 X 14 = 28 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
7 - 2	2x4	DRY	No.2	SPF
1 - 4	2x4	DRY	No.2	SPF
5 - 4	2x3	DRY	No.2	SPF
7 - 5	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMV+p	MT20	2.0	4.0		
3	TMW+w	MT20	2.0	4.0		
4	TMV+p	MT20	2.0	4.0		
5	BMV1+p	MT20	2.0	4.0		
6	BMW1+w	MT20	2.0	4.0		
7	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

PROVIDE ANCHORAGE AT ALL BEARING JOINTS FOR 150 LBS FACTORED UPLIFT.

PROVIDE FOR 155 LBS FACTORED HORIZONTAL REACTION AT JOINT 7

HORIZONTAL REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
7	---	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (11)

CHORDS						WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)		LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM	TO			FR-TO			
7- 2	-184 / 122	0.0	0.0	0.07 (6)	7.81	6- 3	-170 / 145	0.03 (7)	
1- 2	0 / 16	-77.3	-77.3	0.10 (1)	10.00				
2- 3	-68 / 2	-77.3	-77.3	0.04 (1)	6.25				
3- 4	-33 / 30	-77.3	-77.3	0.04 (1)	6.25				
5- 4	-66 / 54	0.0	0.0	0.06 (5)	7.81				
7- 6	-31 / 36	-17.5	-17.5	0.02 (6)	6.25				
6- 5	-27 / 41	-17.5	-17.5	0.02 (11)	6.25				

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF	
	DL =	3.0	PSF	
BOT CH.	LL =	0.0	PSF	
	DL =	7.0	PSF	
TOTAL LOAD	=	33.3	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.10 (1-2:1) , BC=0.02 (6-7:6) , WB=0.03 (3-6:7) , SSI=0.08 (2-7:6)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (3) (INPUT = 0.90)
JSI METAL= 0.08 (7) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
T.L. WISE
100083566
PROVINCE OF ONTARIO

March 10, 2017

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

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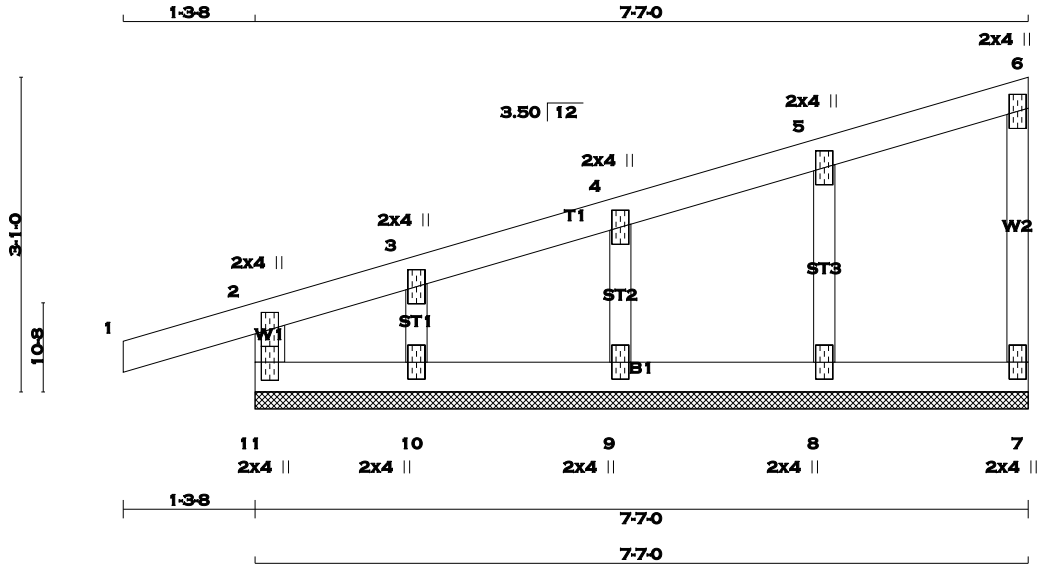


TOTAL WEIGHT = 2 X 25 = 50 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
11 - 2	2x4	DRY	No.2	SPF
1 - 6	2x4	DRY	No.2	SPF
7 - 6	2x3	DRY	No.2	SPF
11 - 7	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMV+p	MT20	2.0	4.0		
3, 4, 5						
3	TMW+w	MT20	2.0	4.0		
6	TMV+p	MT20	2.0	4.0		
7	BMV1+p	MT20	2.0	4.0		
8, 9, 10						
8	BMW1+w	MT20	2.0	4.0		
11	BMV1+p	MT20	2.0	4.0	2.25	1.00

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

PROVIDE ANCHORAGE AT ALL BEARING JOINTS FOR 150 LBS FACTORED UPLIFT.

PROVIDE FOR 203 LBS FACTORED HORIZONTAL REACTION AT JOINT 11

HORIZONTAL REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
11	---	0 / 0	0 / 0	0 / 0	145 / 0	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
11- 2	-194 / 88	0.0	0.0	0.06 (7)	7.81	8- 5	-168 / 132	0.03 (1)
1- 2	0 / 14	-77.3	-77.3	0.10 (1)	10.00	9- 4	-160 / 140	0.03 (5)
2- 3	-124 / 0	-77.3	-77.3	0.07 (1)	6.25	10- 3	-101 / 129	0.03 (7)
3- 4	-95 / 0	-77.3	-77.3	0.04 (1)	6.25			
4- 5	-70 / 22	-77.3	-77.3	0.04 (1)	6.25			
5- 6	-38 / 45	-77.3	-77.3	0.04 (5)	6.25			
7- 6	-66 / 57	0.0	0.0	0.08 (5)	7.81			
11-10	-42 / 48	-17.5	-17.5	0.05 (6)	6.25			
10- 9	-37 / 53	-17.5	-17.5	0.02 (5)	6.25			
9- 8	-35 / 56	-17.5	-17.5	0.02 (11)	6.25			
8- 7	-33 / 59	-17.5	-17.5	0.03 (5)	6.25			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	23.3	PSF
		DL =	3.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.0	PSF
TOTAL	LOAD	=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.10 (1-2:1) , BC=0.05 (10-11:6) , WB=0.03 (4-9:5) , SSI=0.11 (2-11:6)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

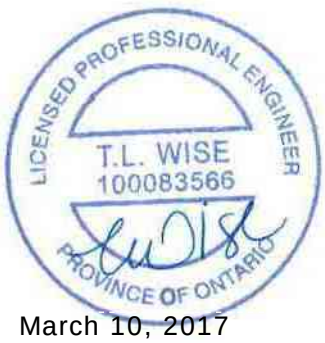
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.16 (11) (INPUT = 0.90)
JSI METAL= 0.10 (11) (INPUT = 1.00)



March 10, 2017

KOTT

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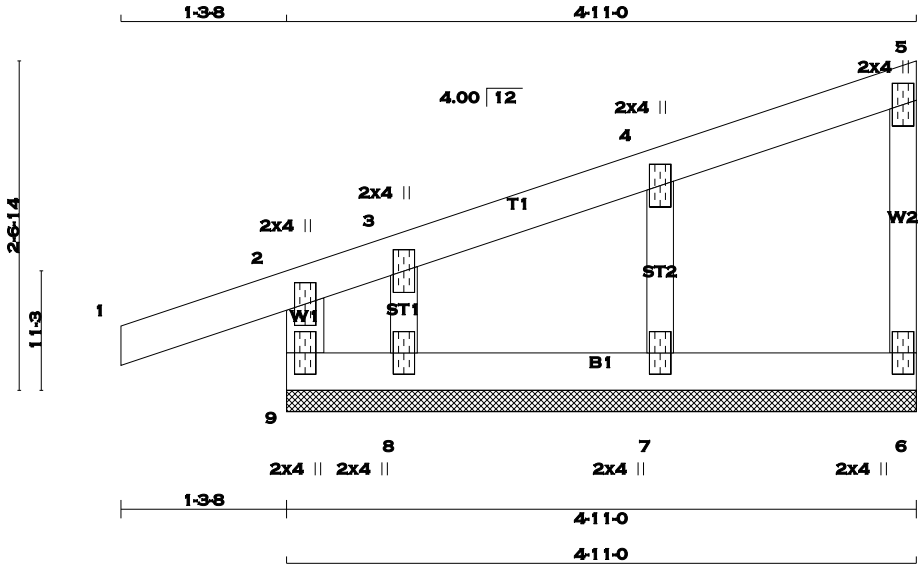


TOTAL WEIGHT = 2 X 17 = 34 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
9 - 2	2x4	DRY	No.2	SPF
1 - 5	2x4	DRY	No.2	SPF
6 - 5	2x3	DRY	No.2	SPF
9 - 6	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMV+p	MT20	2.0	4.0		
3	TMW+w	MT20	2.0	4.0		
4	TMW+w	MT20	2.0	4.0		
5	TMV+p	MT20	2.0	4.0		
6	BMV1+p	MT20	2.0	4.0		
7	BMW1+w	MT20	2.0	4.0		
8	BMW1+w	MT20	2.0	4.0		
9	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

PROVIDE ANCHORAGE AT ALL BEARING JOINTS FOR 150 LBS FACTORED UPLIFT.

PROVIDE FOR 175 LBS FACTORED HORIZONTAL REACTION AT JOINT 9

HORIZONTAL REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
9	---	0 / 0	0 / 0	0 / 0	125 / 0	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
9- 2	-193 / 78	0.0	0.0	0.06 (7)	7.81	7- 4	-181 / 147	0.03 (7)
1- 2	0 / 16	-77.3	-77.3	0.10 (1)	10.00	8- 3	-46 / 104	0.02 (7)
2- 3	-101 / 0	-77.3	-77.3	0.08 (1)	6.25			
3- 4	-72 / 7	-77.3	-77.3	0.05 (1)	6.25			
4- 5	-36 / 35	-77.3	-77.3	0.05 (1)	6.25			
6- 5	-64 / 54	0.0	0.0	0.06 (5)	7.81			
9- 8	-38 / 39	-17.5	-17.5	0.04 (6)	6.25			
8- 7	-34 / 44	-17.5	-17.5	0.02 (11)	6.25			
7- 6	-30 / 48	-17.5	-17.5	0.02 (5)	6.25			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.10 (1-2:1) , BC=0.04 (8-9:6) , WB=0.03 (4-7:7) , SSI=0.09 (2-9:6)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

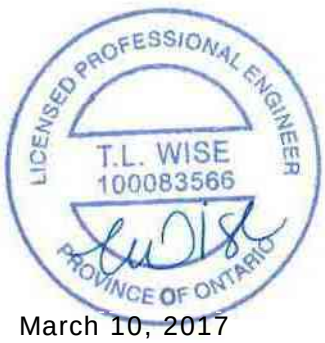
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (4) (INPUT = 0.90)
JSI METAL= 0.09 (9) (INPUT = 1.00)



March 10, 2017

KOTT

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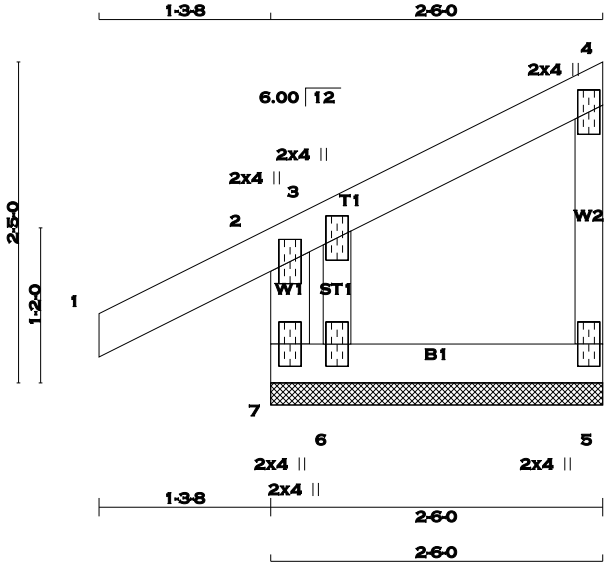


SCALE = 1:17.3

TOTAL WEIGHT = 11 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
7 - 2	2x4	DRY	No.2	SPF
1 - 4	2x4	DRY	No.2	SPF
5 - 4	2x3	DRY	No.2	SPF
7 - 5	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMV+p	MT20	2.0	4.0		
3	TMW+w	MT20	2.0	4.0		
4	TMV+p	MT20	2.0	4.0		
5	BMV1+p	MT20	2.0	4.0		
6	BMW1+w	MT20	2.0	4.0		
7	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

PROVIDE ANCHORAGE AT ALL BEARING JOINTS FOR 150 LBS FACTORED UPLIFT EXCEPT 6:196 LBS.

PROVIDE FOR 154 LBS FACTORED HORIZONTAL REACTION AT JOINT 7

HORIZONTAL REACTIONS

	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
7	---	0 / 0	0 / 0	0 / 0	110 / -49	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (12)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
7- 2	-220 / 45	0.0	0.0	0.05 (7)	7.81	6- 3	-49 / 146	0.03 (6)
1- 2	0 / 23	-77.3	-77.3	0.10 (1)	10.00			
2- 3	-99 / 26	-77.3	-77.3	0.08 (1)	6.25			
3- 4	-39 / 33	-77.3	-77.3	0.04 (1)	6.25			
5- 4	-72 / 58	0.0	0.0	0.06 (7)	7.81			
7- 6	-36 / 43	-17.5	-17.5	0.05 (6)	6.25			
6- 5	-27 / 44	-17.5	-17.5	0.02 (5)	6.25			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.10 (1-2:1) , BC=0.05 (6-7:6) , WB=0.03 (3-6:6) , SSI=0.08 (2-3:12)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

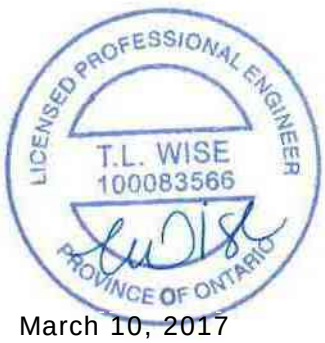
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

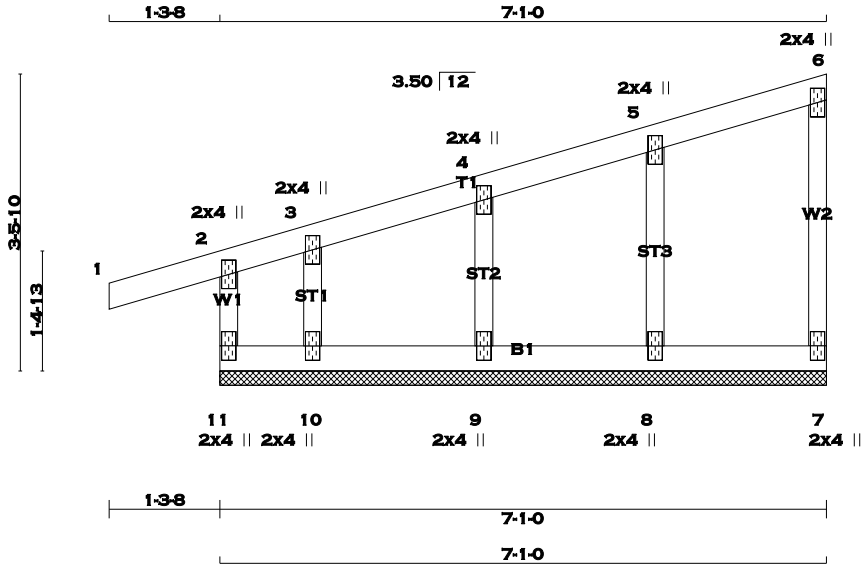
JSI GRIP= 0.14 (3) (INPUT = 0.90)
JSI METAL= 0.08 (7) (INPUT = 1.00)



March 10, 2017

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



SCALE = 1:26.9

TOTAL WEIGHT = 25 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
11 - 2	2x3	DRY	No.2	SPF
1 - 6	2x4	DRY	No.2	SPF
7 - 6	2x3	DRY	No.2	SPF
11 - 7	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMV+p	MT20	2.0	4.0		
3, 4, 5						
3	TMW+w	MT20	2.0	4.0		
6	TMV+p	MT20	2.0	4.0		
7	BMV1+p	MT20	2.0	4.0		
8, 9, 10						
8	BMW1+w	MT20	2.0	4.0		
11	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

PROVIDE ANCHORAGE AT ALL BEARING JOINTS FOR 150 LBS FACTORED UPLIFT EXCEPT 10:231 LBS.

PROVIDE FOR 228 LBS FACTORED HORIZONTAL REACTION AT JOINT 11

HORIZONTAL REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
11	---	0 / 0	0 / 0	0 / 0	163 / -3	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	FORCE (LBS)	MAX CSI (LC)
11- 2	-209 / 68	0.0	0.0	0.20 (6)	7.81	8- 5	-169 / 124	0.03 (1)
1- 2	0 / 14	-77.3	-77.3	0.10 (1)	10.00	9- 4	-162 / 143	0.03 (5)
2- 3	-135 / 0	-77.3	-77.3	0.09 (4)	6.25	10- 3	-54 / 150	0.03 (6)
3- 4	-94 / 6	-77.3	-77.3	0.04 (1)	6.25			
4- 5	-70 / 31	-77.3	-77.3	0.04 (1)	6.25			
5- 6	-38 / 54	-77.3	-77.3	0.06 (5)	6.25			
7- 6	-66 / 61	0.0	0.0	0.11 (5)	7.81			
11-10	-50 / 58	-17.5	-17.5	0.10 (6)	6.25			
10- 9	-46 / 61	-17.5	-17.5	0.02 (5)	6.25			
9- 8	-44 / 64	-17.5	-17.5	0.02 (5)	6.25			
8- 7	-42 / 66	-17.5	-17.5	0.04 (5)	6.25			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.20 (2-11:6) , BC=0.10 (10-11:6) , WB=0.03 (3-10:6) , SSI=0.17 (2-11:6)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (11) (INPUT = 0.90)
JSI METAL= 0.12 (11) (INPUT = 1.00)

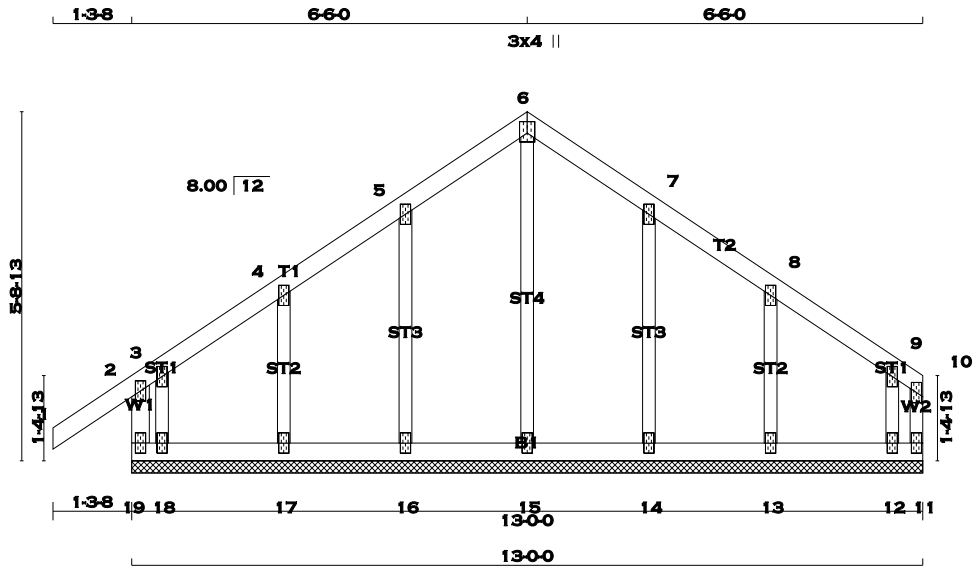
LICENSED PROFESSIONAL ENGINEER
T.L. WISE
100083566
PROVINCE OF ONTARIO

March 10, 2017

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

RECEIVED
TOWN OF MILTON
MAR 29, 2017
17-4978
BUILDING DIVISION



TOTAL WEIGHT = 53 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
19 - 2	2x4	DRY	No.2	SPF
1 - 6	2x4	DRY	No.2	SPF
6 - 10	2x4	DRY	No.2	SPF
11 - 10	2x3	DRY	No.2	SPF
19 - 11	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMV+p	MT20	2.0	4.0		
3, 4, 5, 7, 8, 9						
3	TMW+w	MT20	2.0	4.0		
6	TTW+p	MT20	3.0	4.0	2.25	1.50
10	TMV+p	MT20	2.0	4.0		
11	BMV1+p	MT20	2.0	4.0		
12, 13, 14, 15, 16, 17, 18						
12	BMW1+w	MT20	2.0	4.0		
19	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

PROVIDE ANCHORAGE AT ALL BEARING JOINTS FOR 150 LBS FACTORED UPLIFT EXCEPT 19:288 LBS,11:167 LBS,16:154 LBS,17:158 LBS,18:373 LBS,14:151 LBS,13:164 LBS,12:278 LBS.

PROVIDE FOR 256 LBS FACTORED HORIZONTAL REACTION AT JOINT 19

HORIZONTAL REACTIONS

	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
19	---	0 / 0	0 / 0	0 / 0	183 / -174	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
19- 2	-258 / 138	0.0	0.0	0.10 (6)	7.81	15- 6	-168 / 5	0.08 (1)
1- 2	0 / 29	-77.3	-77.3	0.10 (1)	10.00	16- 5	-188 / 178	0.05 (3)
2- 3	-200 / 172	-77.3	-77.3	0.09 (4)	6.25	17- 4	-184 / 191	0.04 (7)
3- 4	-95 / 126	-77.3	-77.3	0.04 (8)	6.25	18- 3	-98 / 202	0.04 (6)
4- 5	-37 / 183	-77.3	-77.3	0.05 (7)	6.25	14- 7	-186 / 175	0.05 (4)
5- 6	-33 / 238	-77.3	-77.3	0.08 (8)	6.25	13- 8	-187 / 193	0.04 (8)
6- 7	-26 / 221	-77.3	-77.3	0.08 (8)	6.25	12- 9	-160 / 215	0.04 (8)
7- 8	-17 / 147	-77.3	-77.3	0.06 (8)	6.25			
8- 9	-28 / 79	-77.3	-77.3	0.04 (8)	6.25			
9-10	-117 / 89	-77.3	-77.3	0.04 (6)	6.25			
11-10	-136 / 94	0.0	0.0	0.10 (5)	7.81			
19-18	-85 / 115	-17.5	-17.5	0.10 (6)	6.25			
18-17	-73 / 109	-17.5	-17.5	0.03 (5)	6.25			
17-16	-69 / 109	-17.5	-17.5	0.02 (5)	6.25			
16-15	-67 / 109	-17.5	-17.5	0.02 (5)	6.25			
15-14	-67 / 109	-17.5	-17.5	0.02 (5)	6.25			
14-13	-67 / 107	-17.5	-17.5	0.02 (5)	6.25			
13-12	-67 / 104	-17.5	-17.5	0.02 (5)	6.25			
12-11	-63 / 93	-17.5	-17.5	0.06 (5)	6.25			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.10 (1-2:1) , BC=0.10 (18-19:6) , WB=0.08 (6-15:1) , SSI=0.13 (18-19:6)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

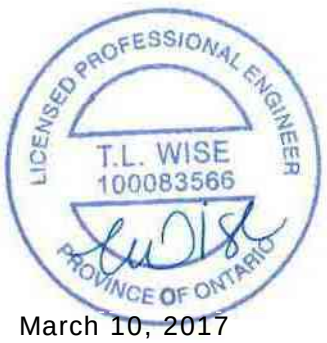
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (6) (INPUT = 0.90)
JSI METAL= 0.16 (6) (INPUT = 1.00)



March 10, 2017

KOTT

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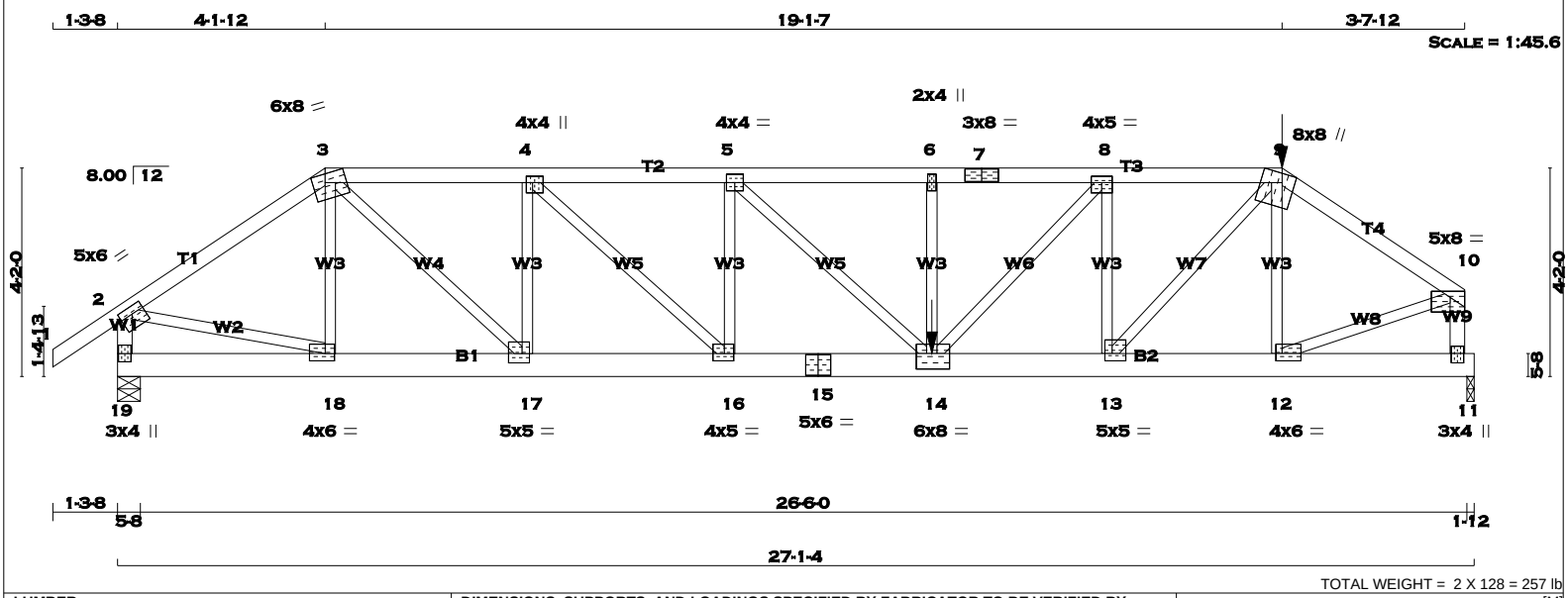


TOTAL WEIGHT = 2 X 128 = 257 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 3	2x4	DRY	No.2	SPF
3 - 7	2x4	DRY	No.2	SPF
7 - 9	2x4	DRY	No.2	SPF
9 - 10	2x4	DRY	No.2	SPF
19 - 2	2x4	DRY	No.2	SPF
11 - 10	2x4	DRY	No.2	SPF
19 - 15	2x6	DRY	No.2	SPF
15 - 11	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)		LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
1- 3	1	12	TOP
3- 7	1	12	SIDE(44.8)
7- 9	1	12	SIDE(44.8)
9- 10	1	12	SIDE(0.0)
19- 2	1	12	TOP
11- 10	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
19- 15	2	12	TOP
15- 11	2	12	SIDE(8.7)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

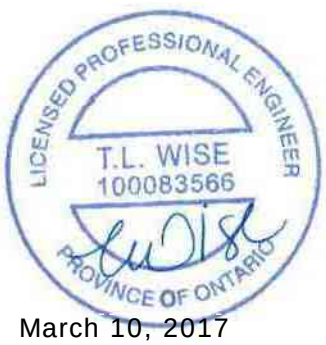
GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-t	MT20	5.0	6.0	1.75	3.00



March 10, 2017

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
19	2072	0	2271	196	-1106	5-8	5-8
11	2716	0	3035	0	-1461	1-12	1-12

PROVIDE ANCHORAGE AT BEARING JOINT 19 FOR 1106 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 11 FOR 1461 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 196 LBS FACTORED HORIZONTAL REACTION AT JOINT 19

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
19	1451	1030 / 0	0 / 0	0 / 0	497 / -1060	421 / 0	0 / 0
11	1907	1329 / 0	0 / 0	0 / 0	797 / -1415	578 / 0	0 / 0
HORIZONTAL REACTIONS							
19	---	0 / 0	0 / 0	0 / 0	142 / -128	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 19, 11

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.96 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
1- 2	0 / 29	-77.3	-77.3	0.06 (1)		10.00
2- 3	-2598 / 1292	-77.3	-77.3	0.23 (7)		5.51
3- 4	-3790 / 1957	-77.3	-77.3	0.21 (7)		4.80
4- 5	-4983 / 2519	-77.3	-77.3	0.26 (7)		4.27
5- 6	-5707 / 2854	-77.3	-77.3	0.27 (8)		4.05
6- 7	-5707 / 2854	-145.8	-145.8	0.32 (8)		3.96
7- 8	-5707 / 2854	-145.8	-145.8	0.32 (8)		3.96
8- 9	-4610 / 2355	-145.8	-145.8	0.28 (8)		4.36
9-10	-3339 / 1669	-77.3	-77.3	0.23 (8)		5.05
19- 2	-2239 / 1127	0.0	0.0	0.11 (1)		7.50
11-10	-2982 / 1497	0.0	0.0	0.15 (1)		6.73
19-18	-177 / 169	-17.5	-17.5	0.02 (11)		6.25
18-17	-1005 / 2163	-17.5	-17.5	0.14 (1)		6.25
17-16	-1801 / 3792	-17.5	-17.5	0.25 (1)		6.25
16-15	-2362 / 4984	-17.5	-17.5	0.36 (1)		6.25
15-14	-2362 / 4984	-17.5	-17.5	0.36 (1)		6.25
14-13	-2164 / 4612	-33.0	-33.0	0.33 (1)		6.25
13-12	-1238 / 2751	-33.0	-33.0	0.17 (1)		6.25
12-11	-12 / 26	-33.0	-33.0	0.03 (11)		6.25

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
18- 3	-407 / 267	0.05 (3)
3-17	-1131 / 2230	0.27 (8)
17- 4	-1473 / 832	0.18 (3)
4-16	-810 / 1636	0.20 (8)
16- 5	-999 / 578	0.12 (3)
5-14	-487 / 996	0.12 (8)
14- 6	-429 / 323	0.05 (3)
14- 8	-772 / 1616	0.18 (1)
13- 8	-1858 / 1060	0.22 (3)
13- 9	-1362 / 2740	0.30 (1)
12- 9	-841 / 542	0.10 (3)
2-18	-953 / 2189	0.25 (1)
12-10	-1357 / 2943	0.32 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
9	23-3-4	-222	-291	202	FRONT	VERT	TOTAL
14	3-	26		6	ONT	VERT	TOTAL

WIND LOADS APPLIED TO THIS TRUSS WERE CALCULATED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} MEAN HEIGHT IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM/COMPONENT AND CLADDING} WIND LOADS MAY BE LOCATED ON {OPEN/ENCLOSED/PARTIALLY ENCLOSED} BUILDING. AT LEAST {1.0} FT-IN-SX AWAY FROM

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP	CH.	LL =	23.3	PSF
		DL =	3.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.0	PSF
TOTAL	LOAD	=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 4-1-12
RIGHT SETBACK = 3-10-0
END SETBACK = 6-0-0
END WALL WIDTH = 5-8
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 10-10-0 OF SPAN MEASURED FROM THE RIGHT.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.90")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.90")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.32 (6-8:8) , BC=0.36 (14-16:1) , WB=0.32 (10-12:1) , SSI=0.13 (8-9:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
3	TTWW-m	MT20	6.0	8.0	2.00	3.00
4	TMWW+t	MT20	4.0	4.0	1.50	1.50
5	TMWW-t	MT20	4.0	4.0		
6	TMW+w	MT20	2.0	4.0		
7	TS-t	MT20	3.0	8.0		
8	TMWW-t	MT20	4.0	5.0	1.50	2.50
9	TTWW+m	MT20	8.0	8.0	Edge	3.00
10	TMVW-p	MT20	5.0	8.0	1.50	Edge
11	BMV1+p	MT20	3.0	4.0	2.25	1.50
12	BMWW-t	MT20	4.0	6.0	1.75	1.50
13	BMWW-t	MT20	5.0	5.0	1.75	1.75
14	BMWWW-t	MT20	6.0	8.0	3.75	2.50
15	BS-t	MT20	5.0	6.0		
16	BMWW-t	MT20	4.0	5.0	1.75	2.25
17	BMWW-t	MT20	5.0	5.0	2.25	1.75
18	BMWW-t	MT20	4.0	6.0	1.75	2.25
19	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

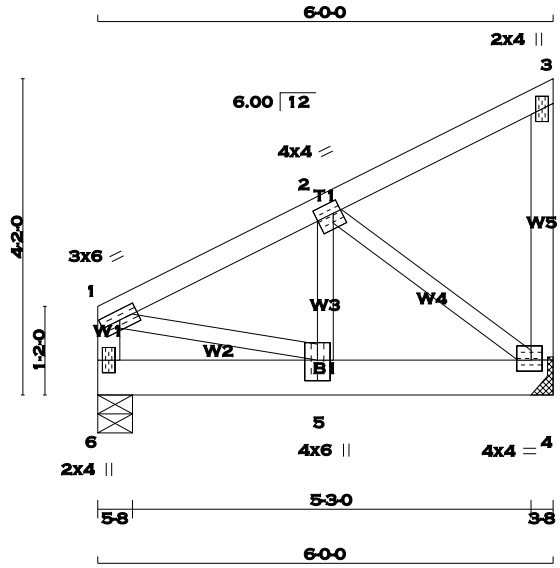
HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 290.7 lbs FACTORED DOWN AND 201.8 lbs FACTORED UP AT 23-3-4 ON TOP CHORD, AND 1441.5 lbs FACTORED DOWN AND 665.9 lbs FACTORED UP AT 16-3-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

JSI GRIP= 0.56 (12) (INPUT = 0.90)
JSI METAL= 0.42 (15) (INPUT = 1.00)

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



SCALE = 1:30.4

TOTAL WEIGHT = 29 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 3	2x4	DRY	No.2	SPF
4 - 3	2x4	DRY	No.2	SPF
6 - 1	2x4	DRY	No.2	SPF
6 - 4	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
1	TMVW-t	MT20	3.0	6.0		
2	TMWW-t	MT20	4.0	4.0	1.75	1.25
3	TMV+p	MT20	2.0	4.0		
4	BMVW1-t	MT20	4.0	4.0	1.75	1.75
5	BMWW+t	MT20	4.0	6.0	3.25	2.00
6	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
4	1263	0	1456	0	-537	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	
6	1263	0	1455	242	-471	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 4 FOR 537 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 6 FOR 471 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 242 LBS FACTORED HORIZONTAL REACTION AT JOINT 6

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
4	886	620 / 0	0 / 0	0 / 0	483 / -555	266 / 0	0 / 0
6	886	620 / 0	0 / 0	0 / 0	481 / -508	266 / 0	0 / 0
HORIZONTAL REACTIONS							
6	---	0 / 0	0 / 0	0 / 0	173 / -70	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 6

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	-1253 / 408	-77.3	-77.3	0.17 (7)	5.66	5- 2	-312 / 1117	0.24 (3)
2- 3	-72 / 71	-77.3	-77.3	0.11 (7)	6.25	2- 4	-1406 / 571	0.31 (3)
4- 3	-102 / 78	0.0	0.0	0.10 (7)	7.81	1- 5	-323 / 1167	0.25 (4)
6- 1	-950 / 335	0.0	0.0	0.09 (1)	7.81			
6- 5	-226 / 91	-343.5	-343.5	0.21 (3)	6.25			
5- 4	-432 / 1122	-343.5	-343.5	0.33 (3)	6.25			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	33.3	PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 16-2-8
END DISTANCE = 6-0-0
END SPAN CARRIED = 16-2-8
END WALL WIDTH = 5-8
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.17 (1-2:7) , BC=0.33 (4-5:3) , WB=0.31 (2-4:3) , SSI=0.40 (5-6:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (5) (INPUT = 0.90)
JSI METAL= 0.42 (2) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
T.L. WISE
100083566
PROVINCE OF ONTARIO
March 10, 2017

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

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MAR 29, 2017
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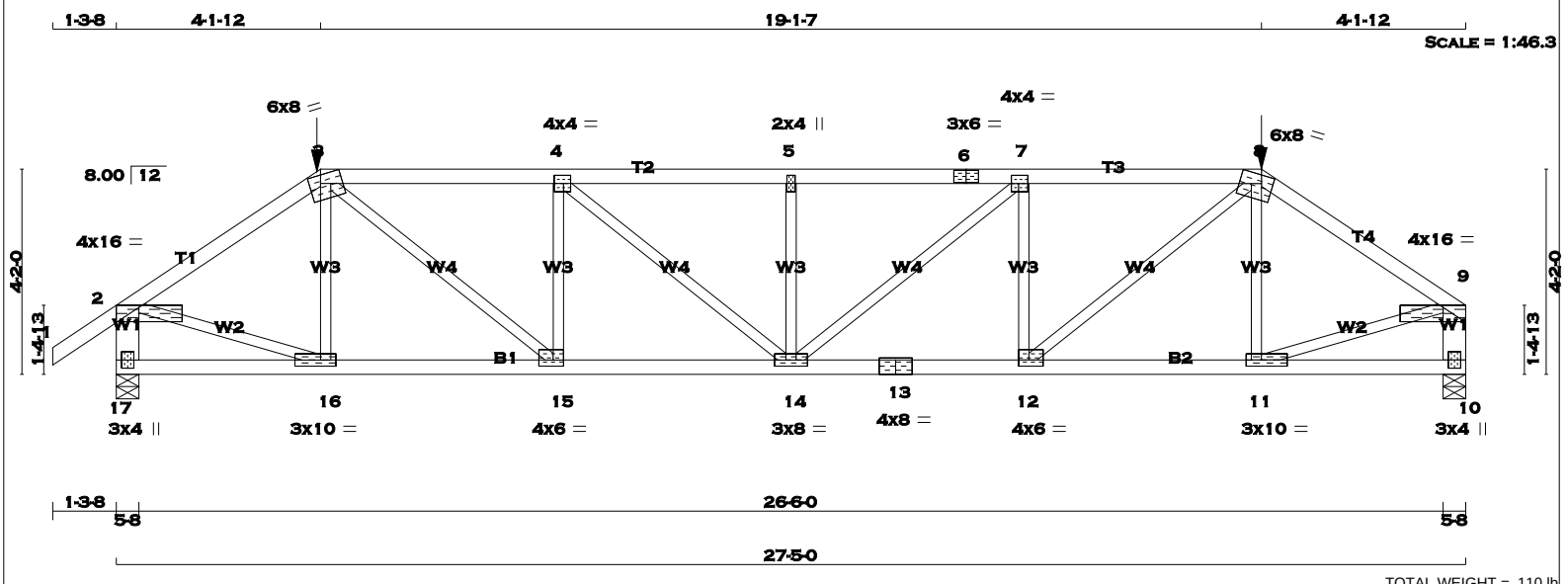

TOTAL WEIGHT = 110 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 3	2x4	DRY	No.2	SPF
3 - 6	2x4	DRY	No.2	SPF
6 - 8	2x4	DRY	No.2	SPF
8 - 9	2x4	DRY	No.2	SPF
17 - 2	2x6	DRY	No.2	SPF
10 - 9	2x6	DRY	No.2	SPF
17 - 13	2x4	DRY	No.2	SPF
13 - 10	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-p	MT20	4.0	16.0	Edge	5.50
3	TTWW-m	MT20	6.0	8.0	2.00	2.75
4	TMWW-t	MT20	4.0	4.0	2.00	1.75
5	TMW+w	MT20	2.0	4.0		
6	TS-t	MT20	3.0	6.0		
7	TMWW-t	MT20	4.0	4.0	2.00	1.75
8	TTWW-m	MT20	6.0	8.0	2.00	2.75
9	TMVW-p	MT20	4.0	16.0	Edge	5.50
10	BMV1+p	MT20	3.0	4.0		
11	BMWW-t	MT20	3.0	10.0	1.50	3.75
12	BMWW-t	MT20	4.0	6.0	1.50	2.50
13	BS-t	MT20	4.0	8.0		
14	BMWWW-t	MT20	3.0	8.0		
15	BMWW-t	MT20	4.0	6.0	1.50	2.50
16	BMWW-t	MT20	3.0	10.0	1.50	3.75
17	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 314.4 lbs FACTORED DOWN AND 218.2 lbs FACTORED UP AT 23-3-4, AND 314.4 lbs FACTORED DOWN AND 218.2 lbs FACTORED UP AT 4-1-12 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
T.L. WISE
100083566
PROVINCE OF ONTARIO
March 10, 2017

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
17	2514	0	2869	195	-1393	5-8	5-8
10	2408	0	2725	0	-1331	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 17 FOR 1393 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 10 FOR 1331 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 195 LBS FACTORED HORIZONTAL REACTION AT JOINT 17

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
17	1763	1241 / 0	0 / 0	0 / 0	888 / -1330	522 / 0	0 / 0
10	1691	1177 / 0	0 / 0	0 / 0	793 / -1281	514 / 0	0 / 0
HORIZONTAL REACTIONS							
17	---	0 / 0	0 / 0	0 / 0	139 / -131	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 17, 10

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.14 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 4.46 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (11)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF) CSI (LC)	MAX. UNBRAC LENGTH
1- 2	0 / 29	-77.3	-77.3	0.11 (1)	10.00
2- 3	-3292 / 1666	-77.3	-77.3	0.52 (7)	3.52
3- 4	-4624 / 2417	-145.8	-145.8	0.93 (3)	2.39
4- 5	-5164 / 2658	-145.8	-145.8	1.00 (3)	2.14
5- 6	-5164 / 2658	-145.8	-145.8	1.00 (3)	2.14
6- 7	-5164 / 2658	-145.8	-145.8	1.00 (3)	2.14
7- 8	-4607 / 2410	-145.8	-145.8	0.92 (3)	2.40
8- 9	-3270 / 1654	-77.3	-77.3	0.52 (8)	3.56
17- 2	-2814 / 1432	0.0	0.0	0.18 (1)	6.26
10- 9	-2670 / 1370	0.0	0.0	0.17 (1)	6.39
17-16	-174 / 173	-33.0	-33.0	0.17 (11)	6.25
16-15	-1308 / 2737	-33.0	-33.0	0.51 (1)	5.57
15-14	-2257 / 4626	-33.0	-33.0	0.80 (1)	4.46
14-13	-2216 / 4609	-33.0	-33.0	0.80 (1)	4.50
13-12	-2216 / 4609	-33.0	-33.0	0.80 (1)	4.50
12-11	-1226 / 2703	-33.0	-33.0	0.51 (1)	5.71
11-10	-10 / 21	-33.0	-33.0	0.17 (11)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
16- 3	-519 / 432	0.13 (3)
3-15	-1264 / 2418	0.81 (8)
15- 4	-1358 / 901	0.35 (3)
4-14	-359 / 701	0.23 (8)
14- 5	-744 / 543	0.19 (3)
14- 7	-364 / 720	0.23 (7)
12- 7	-1372 / 904	0.35 (3)
12- 8	-1268 / 2439	0.81 (7)
11- 8	-521 / 437	0.13 (3)
2-16	-1278 / 2794	0.61 (1)
11- 9	-1299 / 2804	0.61 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
3	4-1-12	-241	-314	218	FRONT	VERT	TOTAL
8	23-3-4	-241	-314	218	FRONT	VERT	TOTAL

WIND LOADS APPLIED AS PRESSURE FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0 FROM LEFT END TO RIGHT END} MEAN HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, AS PER SECTION {MAIN WIND FORCE RESISTING SYSTEM} INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0.00} FT FROM ENDS AWAY FRO

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

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17-4978
BUILDING DIVISION

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 4-1-12
END SETBACK = 6-0-0
END WALL WIDTH = 5-8
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.91")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.26")
ALLOWABLE DEFL.(TL)= L/360 (0.91")
CALCULATED VERT. DEFL.(TL) = L/ 786 (0.42")

CSI: TC=1.00 (5-7:3) , BC=0.80 (12-14:1) , WB=0.81 (8-12:7) , SSI=0.37 (7-8:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

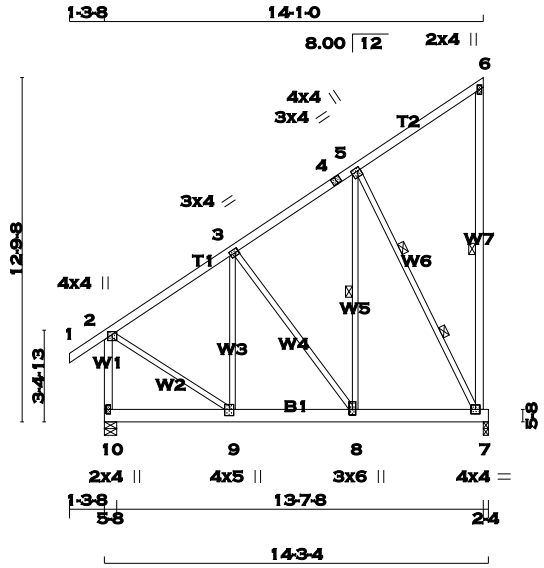
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (15) (INPUT = 0.90)
JSI METAL= 0.93 (13) (INPUT = 1.00)



SCALE = 1:85.6

TOTAL WEIGHT = 6 X 94 = 563 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 6	2x4	DRY	No.2	SPF
7 - 6	2x4	DRY	No.2	SPF
10 - 2	2x4	DRY	No.2	SPF
10 - 7	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)		LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
1- 4	1	12	TOP
4- 6	1	12	TOP
6- 7	1	12	TOP
10- 2	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
10- 7	2	6	SIDE(471.5)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

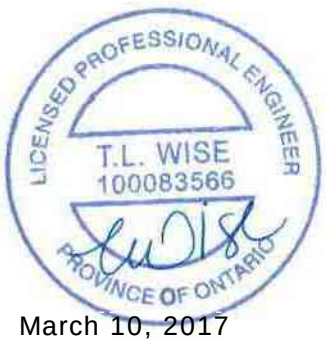
STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW+p	MT20	4.0	4.0	1.25	2.00
3	TMWW-t	MT20	3.0	4.0	1.50	1.50
4	TS-t	MT20	3.0	4.0		
5	TMWW+t	MT20	4.0	4.0	1.50	1.00
6	TMV+p	MT20	2.0	4.0		
7	BMVW1-t	MT20	4.0	4.0		
8	BMWW+t	MT20	3.0	6.0	2.50	1.50
9	BMWW+t	MT20	4.0	5.0	2.75	1.75
10	BMV1+p	MT20	2.0	4.0		



March 10, 2017

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
7	4320	0	5087	0	-1832	2-4	2-4
10	4725	0	5490	761	-1689	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 7 FOR 1832 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 10 FOR 1689 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 761 LBS FACTORED HORIZONTAL REACTION AT JOINT 10

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
7	3032	2121 / 0	0 / 0	0 / 0	1917 / -1894	911 / 0	0 / 0
10	3314	2332 / 0	0 / 0	0 / 0	1911 / -1838	982 / 0	0 / 0
HORIZONTAL REACTIONS							
10	---	0 / 0	0 / 0	0 / 0	544 / -377	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 7, 10

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.85 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 6-7. DBS = 20-0-0 . CBF = 19 LBS.
1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 5-8. DBS = 6-0-0 . CBF = 73 LBS.
2 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 3 LENGTH OF 5-7. DBS = 4-0-0 . CBF = 90 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) TO EACH PLY USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 29	-77.3	-77.3	0.03 (1)	10.00	9- 3	-641 / 1682	0.14 (6)
2- 3	-3502 / 1127	-77.3	-77.3	0.13 (7)	5.85	3- 8	-1776 / 815	0.68 (3)
3- 4	-2325 / 819	-77.3	-77.3	0.12 (7)	6.25	8- 5	-1496 / 4464	0.29 (3)
4- 5	-2325 / 819	-77.3	-77.3	0.12 (7)	6.25	5- 7	-4145 / 1654	0.94 (3)
5- 6	-202 / 287	-77.3	-77.3	0.09 (7)	6.25	2- 9	-1048 / 3465	0.23 (4)
7- 6	-176 / 181	0.0	0.0	0.30 (5)	6.25			
10- 2	-4049 / 1278	0.0	0.0	0.23 (4)	7.03			
10- 9	-703 / 501	-620.2	-620.2	0.28 (3)	6.25			
9- 8	-1237 / 2959	-525.3	-525.3	0.36 (3)	6.25			
8- 7	-784 / 1904	-525.3	-525.3	0.30 (3)	6.25			

WIND LOAD APPLIED USING REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0 FT} MEAN ROOF HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM} INTERNAL WIND PRESSURE COEFFICIENTS BASED ON {ENCLOSED} DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0.0 FT} IN {SIDEWAY} AWAY FROM EAVE.

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

KOTT

RECEIVED
TOWN OF MILTON
MAR 29, 2017
17-4978
BUILDING DIVISION

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 27-10-8
END DISTANCE = 4-7-8
END SPAN CARRIED = 27-10-8
END WALL WIDTH = 5-8
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

GIRDER TYPE: CStdGirder
START DISTANCE = 4-7-8
START SPAN CARRIED = 23-10-8
END DISTANCE = 14-3-4
END SPAN CARRIED = 23-10-8
END WALL WIDTH = 5-8
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.47")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.47")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.30 (6-7:5) , BC=0.36 (8-9:3) , WB=0.94 (5-7:3) , SSI=0.35 (9-10:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

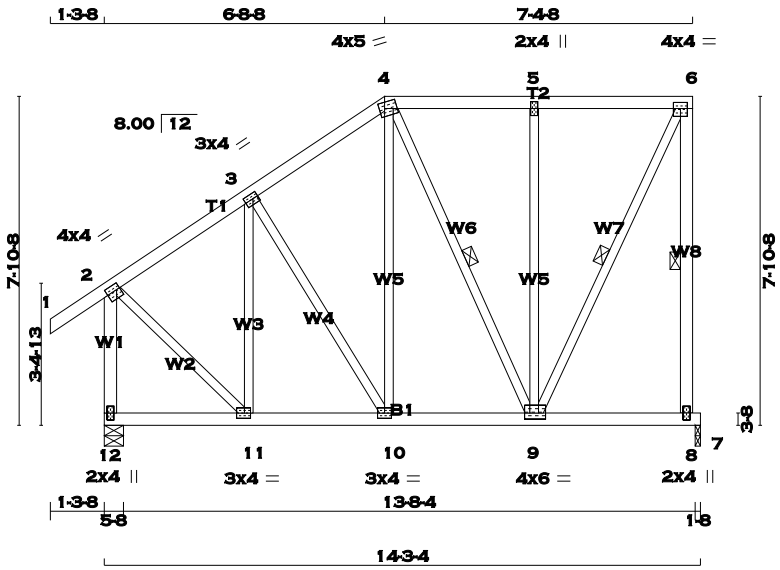
PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (7) (INPUT = 0.90)
JSI METAL= 0.33 (8) (INPUT = 1.00)

CONTINUED ON PAGE 2



SCALE = 1:55.2

TOTAL WEIGHT = 2 X 85 = 170 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 6	2x4	DRY	No.2	SPF
8 - 6	2x4	DRY	No.2	SPF
12 - 2	2x4	DRY	No.2	SPF
12 - 7	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-t	MT20	4.0	4.0	1.75	1.00
3	TMWW-t	MT20	3.0	4.0	1.50	1.50
4	TTWW-m	MT20	4.0	5.0	1.75	1.50
5	TMW+w	MT20	2.0	4.0		
6	TMVW-t	MT20	4.0	4.0	1.75	2.00
8	BMV+p	MT20	2.0	4.0		
9	BMWWW-t	MT20	4.0	6.0	1.75	1.50
10	BMWW-t	MT20	3.0	4.0		
11	BMWW-t	MT20	3.0	4.0	1.50	1.75
12	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
12	1044	0	1134	442	-457	5-8	5-8
7	923	0	1029	0	-498	1-8	1-8

PROVIDE ANCHORAGE AT BEARING JOINT 12 FOR 457 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 7 FOR 498 LBS FACTORED UPLIFT

PROVIDE FOR 442 LBS FACTORED HORIZONTAL REACTION AT JOINT 12

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
12	730	524 / 0	0 / 0	0 / 0	225 / -459	206 / 0	0 / 0
7	648	452 / 0	0 / 0	0 / 0	264 / -482	197 / 0	0 / 0
HORIZONTAL REACTIONS							
12	---	0 / 0	0 / 0	0 / 0	316 / -239	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 12, 7

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 6-8. DBS = 16-0-0 . CBF = 89 LBS.
1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 4-9, 6-9. DBS = 20-0-0 . CBF = 54 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 29	-77.3	-77.3	0.11 (1)	10.00	11- 3	-234 / 108	0.12 (4)
2- 3	-643 / 354	-77.3	-77.3	0.22 (7)	6.25	3-10	-141 / 233	0.10 (3)
3- 4	-623 / 432	-77.3	-77.3	0.23 (7)	6.25	10- 4	-190 / 284	0.22 (7)
4- 5	-441 / 365	-77.3	-77.3	0.18 (7)	6.25	4- 9	-185 / 159	0.09 (3)
5- 6	-441 / 366	-77.3	-77.3	0.18 (7)	6.25	9- 5	-365 / 283	0.42 (3)
8- 6	-988 / 527	0.0	0.0	0.45 (7)	6.25	9- 6	-486 / 969	0.23 (7)
12- 2	-1050 / 455	0.0	0.0	0.20 (1)	7.65	2-11	-167 / 711	0.16 (1)
12-11	-382 / 306	-54.1	-54.1	0.10 (3)	6.25			
11-10	-395 / 580	-54.1	-54.1	0.18 (1)	6.25			
10- 9	-300 / 510	-54.1	-54.1	0.25 (1)	6.25			
9- 8	-58 / 149	-54.1	-54.1	0.27 (1)	6.25			
8- 7	0 / 0	-54.1	-54.1	0.26 (1)	10.00			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, {[illegible] WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN} AND TRUSSES DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	23.3	PSF
		DL =	3.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.0	PSF
TOTAL	LOAD	=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 4-0-0
END DISTANCE = 14-3-4
END SPAN CARRIED = 4-0-0
END WALL WIDTH = 5-8
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.48")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.48")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.45 (6-8:7) , BC=0.27 (8-9:1) , WB=0.42 (5-9:3) , SSI=0.79 (7-8:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

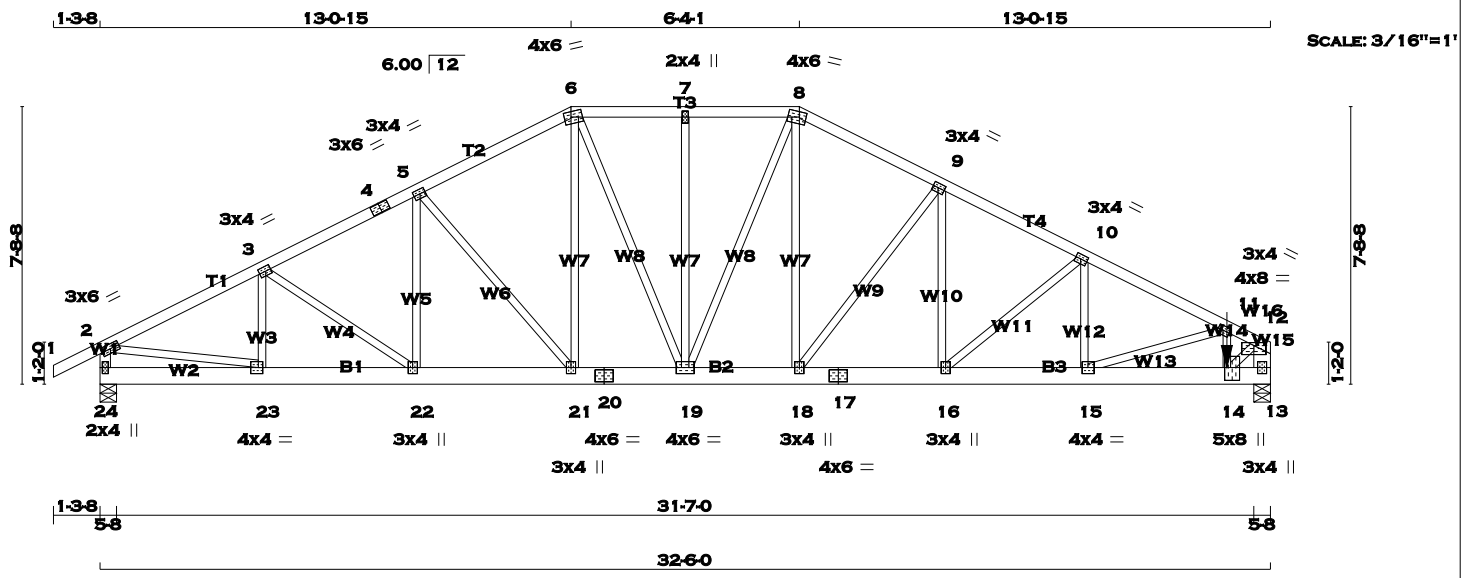
JSI GRIP= 0.89 (11) (INPUT = 0.90)
JSI METAL= 0.29 (2) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
T.L. WISE
100083566
PROVINCE OF ONTARIO
March 10, 2017

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

RECEIVED
TOWN OF MILTON
MAR 29, 2017
17-4978
BUILDING DIVISION



TOTAL WEIGHT = 2 X 171 = 341 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 6	2x4	DRY	No.2	SPF
6 - 8	2x4	DRY	No.2	SPF
8 - 12	2x4	DRY	No.2	SPF
24 - 2	2x4	DRY	No.2	SPF
13 - 12	2x6	DRY	No.2	SPF
24 - 20	2x6	DRY	No.2	SPF
20 - 17	2x6	DRY	No.2	SPF
17 - 13	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

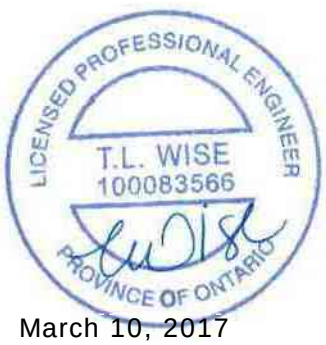
CHORDS #ROWS		SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
1- 4	1	12	TOP
4- 6	1	12	TOP
6- 8	1	12	TOP
8- 12	1	12	TOP
24- 2	1	12	TOP
13- 12	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
24- 20	2	12	TOP
20- 17	2	12	TOP
17- 13	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.



March 10, 2017

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
24	1783	0	1850	207	-857	5-8	5-8
13	5073	0	5716	0	-2586	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 24 FOR 857 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 13 FOR 2586 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 207 LBS FACTORED HORIZONTAL REACTION AT JOINT 24

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
24	1249	887 / 0	0 / 0	0 / 0	168 / -845	362 / 0	0 / 0
13	3560	2490 / 0	0 / 0	0 / 0	1609 / -2535	1070 / 0	0 / 0
HORIZONTAL REACTIONS							
24	---	0 / 0	0 / 0	0 / 0	148 / -121	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 24, 13

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.29 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
1- 2	0 / 23	-77.3	-77.3	0.06 (1)		10.00
2- 3	-2553 / 1182	-77.3	-77.3	0.19 (7)		5.60
3- 4	-2535 / 1234	-77.3	-77.3	0.19 (7)		5.62
4- 5	-2535 / 1234	-77.3	-77.3	0.19 (7)		5.62
5- 6	-2224 / 1139	-77.3	-77.3	0.18 (7)		5.90
6- 7	-2124 / 1118	-77.3	-77.3	0.13 (7)		6.05
7- 8	-2124 / 1118	-77.3	-77.3	0.13 (7)		6.05
8- 9	-2360 / 1211	-77.3	-77.3	0.17 (8)		5.80
9-10	-2953 / 1437	-77.3	-77.3	0.19 (8)		5.32
10-11	-3766 / 1756	-77.3	-77.3	0.20 (8)		4.82
11-12	-5114 / 2332	-77.3	-77.3	0.20 (8)		4.29
24- 2	-1795 / 870	0.0	0.0	0.10 (1)		7.81
13-12	-5278 / 2380	0.0	0.0	0.17 (1)		6.43
24-23	-192 / 169	-17.5	-17.5	0.03 (1)		6.25
23-22	-1135 / 2328	-17.5	-17.5	0.17 (1)		6.25
22-21	-1018 / 2292	-17.5	-17.5	0.15 (1)		6.25
21-20	-777 / 1996	-17.5	-17.5	0.13 (1)		6.25
20-19	-777 / 1996	-17.5	-17.5	0.13 (1)		6.25
19-18	-762 / 2118	-17.5	-17.5	0.14 (1)		6.25
18-17	-1013 / 2647	-17.5	-17.5	0.18 (1)		6.25
17-16	-1013 / 2647	-17.5	-17.5	0.18 (1)		6.25
16-15	-1439 / 3385	-17.5	-17.5	0.23 (1)		6.25
15-14	-2046 / 4566	-17.5	-17.5	0.43 (1)		6.25
14-13	-8 / 16	-17.5	-17.5	0.17 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
23- 3	-347 / 243	0.03 (1)
3-22	-45 / 143	0.02 (7)
22- 5	-34 / 115	0.02 (11)
5-21	-466 / 376	0.19 (3)
21- 6	-239 / 421	0.12 (7)
6-19	-284 / 360	0.18 (8)
19- 7	-300 / 235	0.15 (3)
19- 8	-128 / 91	0.08 (7)
18- 8	-423 / 784	0.22 (8)
18- 9	-892 / 582	0.36 (3)
16- 9	-287 / 650	0.07 (1)
16-10	-967 / 558	0.21 (3)
15-10	-244 / 662	0.07 (3)
15-11	-1251 / 643	0.18 (3)
14-11	-399 / 865	0.09 (3)
2-23	-957 / 2320	0.28 (1)
14-12	-2448 / 5423	0.60 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE TOTAL
14	31-3-8	-3668	-4324	1934	FRONT	VERT	

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF {20} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}. INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP	CH.	LL =	23.3	PSF
		DL =	3.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.0	PSF
TOTAL	LOAD	=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.08")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (1.08")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.20 (10-11:8) , BC=0.43 (14-15:1) , WB=0.60 (12-14:1) , SSI=0.13 (13-14:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (14) (INPUT = 0.90)
JSI METAL= 0.59 (14) (INPUT = 1.00)

CONTINUED ON PAGE 2

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-t	MT20	3.0	6.0		
3, 5, 9, 10, 11						
3	TMWW-t	MT20	3.0	4.0	1.50	1.75
4	TS-t	MT20	3.0	6.0		
6	TTWW-m	MT20	4.0	6.0	1.75	2.25
7	TMW+w	MT20	2.0	4.0		
8	TTWW-m	MT20	4.0	6.0	1.75	2.25
12	TMVW-p	MT20	4.0	8.0	1.00	4.00
13	BMV1+p	MT20	3.0	4.0		
14	BMWW+t	MT20	5.0	8.0	4.25	2.00
15	BMWW-t	MT20	4.0	4.0		
16, 18, 21, 22						
16	BMWW+t	MT20	3.0	4.0		
17	BS-t	MT20	4.0	6.0		
19	BMWWW-t	MT20	4.0	6.0		
20	BS-t	MT20	4.0	6.0		
23	BMWW-t	MT20	4.0	4.0	2.00	1.50
24	BMV1+p	MT20	2.0	4.0		

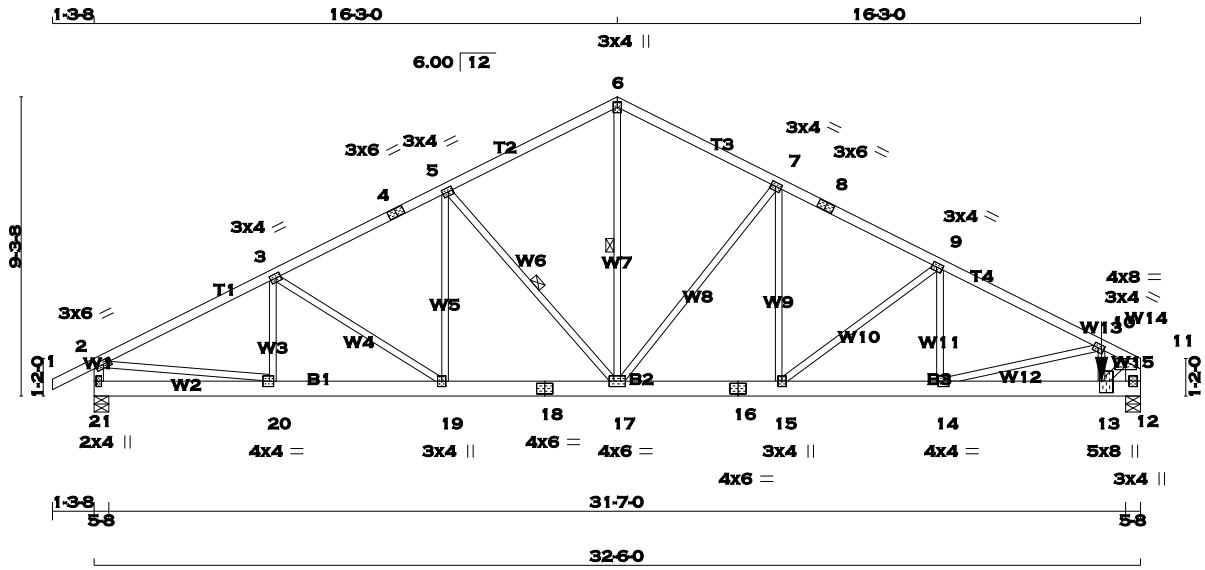
A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 4323.5 lbs FACTORED DOWN AND 1934.4 lbs FACTORED UP AT 31-3-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 6	2x4	DRY	No.2	SPF
6 - 8	2x4	DRY	No.2	SPF
8 - 11	2x4	DRY	No.2	SPF
21 - 2	2x4	DRY	No.2	SPF
12 - 11	2x6	DRY	No.2	SPF
21 - 18	2x6	DRY	No.2	SPF
18 - 16	2x6	DRY	No.2	SPF
16 - 12	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

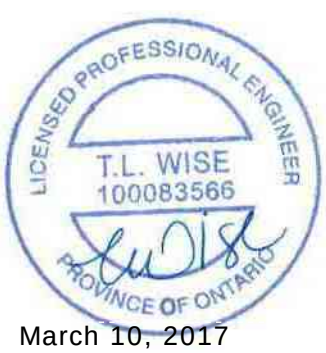
CHORDS #ROWS		SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
1- 4	1	12	TOP
4- 6	1	12	TOP
6- 8	1	12	TOP
8- 11	1	12	TOP
21- 2	1	12	TOP
12- 11	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
21- 18	2	12	TOP
18- 16	2	12	TOP
16- 12	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.



March 10, 2017

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
21	1773	0	1829	248	-805	5-8	5-8
12	4831	0	5424	0	-2411	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 21 FOR 805 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 12 FOR 2411 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 248 LBS FACTORED HORIZONTAL REACTION AT JOINT 21

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
21	1242	882 / 0	0 / 0	0 / 0	138 / -806	360 / 0	0 / 0
12	3391	2372 / 0	0 / 0	0 / 0	1483 / -2377	1019 / 0	0 / 0
HORIZONTAL REACTIONS							
21	---	0 / 0	0 / 0	0 / 0	177 / -143	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 21, 12

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.37 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 5-17, 6-17. DBS = 20-0-0 . CBF = 83 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) TO EACH PLY USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI
1- 2	0 / 23	-77.3	-77.3	0.06	(1)	10.00	20- 3	-243 / 211	0.03 (1)
2- 3	-2584 / 1117	-77.3	-77.3	0.24	(7)	5.50	3-19	-256 / 275	0.09 (3)
3- 4	-2377 / 1088	-77.3	-77.3	0.23	(7)	5.68	19- 5	-87 / 233	0.03 (7)
4- 5	-2377 / 1088	-77.3	-77.3	0.23	(7)	5.68	5-17	-659 / 509	0.15 (3)
5- 6	-1927 / 969	-77.3	-77.3	0.22	(7)	6.15	17- 6	-652 / 1381	0.17 (8)
6- 7	-1933 / 973	-77.3	-77.3	0.20	(8)	6.18	17- 7	-982 / 667	0.67 (4)
7- 8	-2586 / 1192	-77.3	-77.3	0.22	(8)	5.54	15- 7	-285 / 640	0.10 (8)
8- 9	-2586 / 1192	-77.3	-77.3	0.22	(8)	5.54	15- 9	-980 / 603	0.34 (3)
9-10	-3445 / 1530	-77.3	-77.3	0.23	(8)	4.96	14- 9	-199 / 585	0.06 (3)
10-11	-4901 / 2193	-77.3	-77.3	0.23	(8)	4.37	14-10	-1347 / 750	0.30 (3)
21- 2	-1773 / 825	0.0	0.0	0.09	(1)	7.81	13-10	-357 / 774	0.08 (3)
12-11	-5034 / 2212	0.0	0.0	0.16	(1)	6.54	2-20	-879 / 2340	0.28 (1)
							13-11	-2323 / 5220	0.58 (1)
21-20	-233 / 210	-17.5	-17.5	0.03	(8)	6.25			
20-19	-1104 / 2368	-17.5	-17.5	0.17	(1)	6.25			
19-18	-875 / 2155	-17.5	-17.5	0.16	(1)	6.25			
18-17	-875 / 2155	-17.5	-17.5	0.16	(1)	6.25			
17-16	-790 / 2321	-17.5	-17.5	0.17	(1)	6.25			
16-15	-790 / 2321	-17.5	-17.5	0.17	(1)	6.25			
15-14	-1218 / 3096	-17.5	-17.5	0.22	(1)	6.25			
14-13	-1941 / 4395	-17.5	-17.5	0.40	(1)	6.25			
13-12	-8 / 16	-17.5	-17.5	0.15	(3)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
13	31-3-8	-3417	-4028	1802	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP	CH.	LL =	23.3	PSF
		DL =	3.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.0	PSF
TOTAL	LOAD	=	33.3	PSF

SPACING = 24.0 IN. C/C

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.08")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (1.08")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.24 (2-3:7) , BC=0.40 (13-14:1) , WB=0.67 (7-17:4) , SSI=0.11 (12-13:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (2) (INPUT = 0.90)
JSI METAL= 0.57 (13) (INPUT = 1.00)

CONTINUED ON PAGE 2

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-t	MT20	3.0	6.0		
3, 5, 7, 9, 10						
3	TMWW-t	MT20	3.0	4.0	1.50	1.75
4	TS-t	MT20	3.0	6.0		
6	TTW+p	MT20	3.0	4.0		
8	TS-t	MT20	3.0	6.0		
11	TMVW-p	MT20	4.0	8.0	1.00	4.00
12	BMV1+p	MT20	3.0	4.0		
13	BMWW+t	MT20	5.0	8.0	4.25	2.00
14	BMWW-t	MT20	4.0	4.0		
15	BMWW+t	MT20	3.0	4.0		
16	BS-t	MT20	4.0	6.0		
17	BMWWW-t	MT20	4.0	6.0		
18	BS-t	MT20	4.0	6.0		
19	BMWW+t	MT20	3.0	4.0		
20	BMWW-t	MT20	4.0	4.0	2.00	1.50
21	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

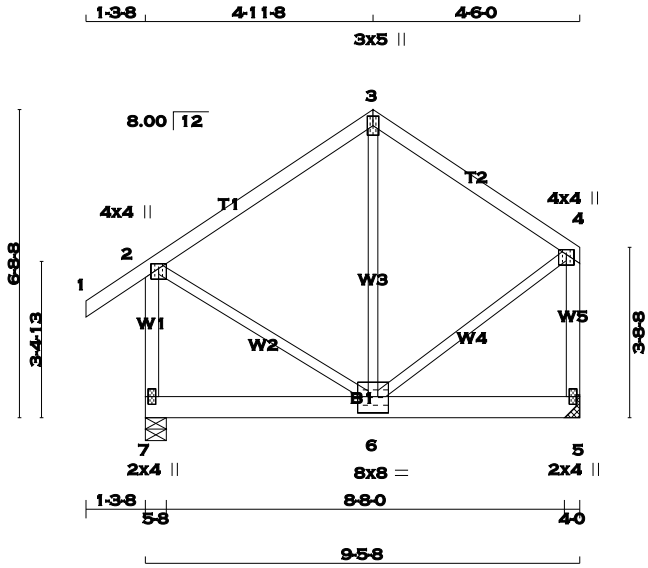
HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 4027.9 lbs FACTORED DOWN AND 1802.2 lbs FACTORED UP AT 31-3-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



TOTAL WEIGHT = 2 X 52 = 104 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 3	2x4	DRY	No.2	SPF
3 - 4	2x4	DRY	No.2	SPF
7 - 2	2x4	DRY	No.2	SPF
5 - 4	2x4	DRY	No.2	SPF
7 - 5	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)		LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
1- 3	1	12	TOP
3- 4	1	12	TOP
7- 2	1	12	TOP
5- 4	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
7- 5	2	8	SIDE(399.4)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

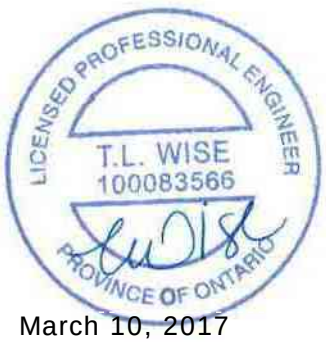
TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW+p	MT20	4.0	4.0	1.25	2.00
3	TTW+p	MT20	3.0	5.0		
4	TMVW+p	MT20	4.0	4.0	1.25	2.00
5	BMV1+p	MT20	2.0	4.0		
6	BMWWW-t	MT20	8.0	8.0	4.25	4.00
7	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.



March 10, 2017

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
7	3774	0	4359	333	-1380	5-8	5-8
5	3668	0	4264	0	-1370	HANGER BY OTHERS MIN. SEAT SIZE: 4-0	

PROVIDE ANCHORAGE AT BEARING JOINT 7 FOR 1380 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 5 FOR 1370 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 333 LBS FACTORED HORIZONTAL REACTION AT JOINT 7

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
7	2646	1865 / 0	0 / 0	0 / 0	1463 / -1488	782 / 0	0 / 0
5	2574	1801 / 0	0 / 0	0 / 0	1491 / -1476	773 / 0	0 / 0
HORIZONTAL REACTIONS							
7	---	0 / 0	0 / 0	0 / 0	238 / -224	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 7

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.68 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 29	-77.3	-77.3	0.06 (1)	10.00	6- 3	-681 / 2196	0.24 (4)
2- 3	-2339 / 834	-77.3	-77.3	0.25 (7)	5.68	2- 6	-738 / 2283	0.25 (4)
3- 4	-2340 / 849	-77.3	-77.3	0.21 (8)	5.73	6- 4	-777 / 2416	0.26 (3)
7- 2	-2764 / 922	0.0	0.0	0.24 (4)	6.92			
5- 4	-2899 / 978	0.0	0.0	0.30 (3)	6.81			
7- 6	-274 / 286	-698.2	-698.2	0.68 (3)	6.25			
6- 5	-30 / 65	-698.2	-698.2	0.68 (4)	6.25			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 31-2-0
END DISTANCE = 9-5-8
END SPAN CARRIED = 31-2-0
END WALL WIDTH = 5-8
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.32")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.32")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.30 (4-5:3) , BC=0.68 (6-7:3) , WB=0.26 (4-6:3) , SSI=0.71 (6-7:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

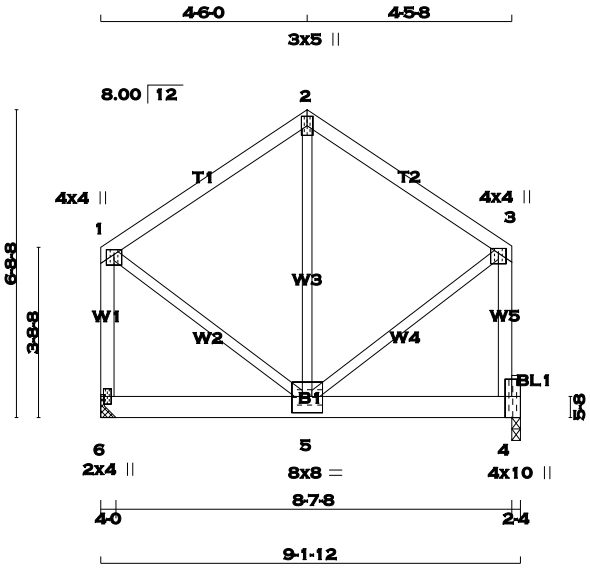
PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (4) (INPUT = 0.90)
JSI METAL= 0.30 (6) (INPUT = 1.00)

CONTINUED ON PAGE 2



TOTAL WEIGHT = 2 X 50 = 99 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 2	2x4	DRY	No.2	SPF
2 - 3	2x4	DRY	No.2	SPF
6 - 1	2x4	DRY	No.2	SPF
4 - 3	2x4	DRY	No.2	SPF
6 - 4	2x6	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)		LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
1- 2	1	12	TOP
2- 3	1	12	TOP
6- 1	1	12	TOP
4- 3	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
6- 4	2	8	SIDE(399.4)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

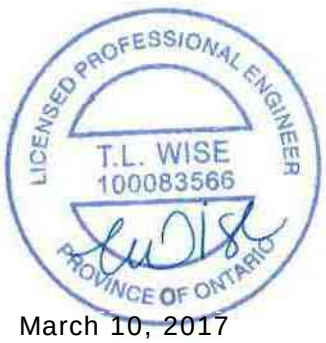
NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
1	TMVW+p	MT20	4.0	4.0	1.25	2.00
2	TTW+p	MT20	3.0	5.0		
3	TMVW+p	MT20	4.0	4.0	1.25	2.00
4	BMVK1+t	MT20	4.0	10.0	Edge	1.75
5	BMWWW-t	MT20	8.0	8.0	4.25	4.00
6	BMV1+p	MT20	2.0	4.0		



March 10, 2017

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
6	3417	0	3974	308	-1261	HANGER BY OTHERS MIN. SEAT SIZE: 4-0	
4	3429	0	3987	0	-1263	2-4	2-4

PROVIDE ANCHORAGE AT BEARING JOINT 6 FOR 1261 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 4 FOR 1263 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 308 LBS FACTORED HORIZONTAL REACTION AT JOINT 6

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
6	2398	1678 / 0	0 / 0	0 / 0	1393 / -1364	721 / 0	0 / 0
4	2406	1685 / 0	0 / 0	0 / 0	1395 / -1366	721 / 0	0 / 0
HORIZONTAL REACTIONS							
6	---	0 / 0	0 / 0	0 / 0	220 / -220	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 4
BEARING SIZE FACTOR = 1.08 AT JNT(S) 4 (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.04 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	-2032 / 739	-77.3	-77.3	0.21 (7)	6.04	5- 2	-586 / 1875	0.20 (4)
2- 3	-2031 / 739	-77.3	-77.3	0.20 (8)	6.04	1- 5	-684 / 2098	0.23 (4)
6- 1	-2541 / 849	0.0	0.0	0.26 (4)	7.16	5- 3	-729 / 2255	0.24 (3)
4- 3	-2649 / 882	0.0	0.0	0.28 (3)	7.05			
6- 5	-244 / 277	-698.2	-698.2	0.58 (3)	6.25			
5- 4	-142 / 104	-698.2	-698.2	0.58 (4)	6.25			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 31-2-0
END DISTANCE = 9-1-2
END SPAN CARRIED = 31-2-0
END WALL WIDTH = 5-8
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.29")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.29")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.28 (3-4:3) , BC=0.58 (5-6:3) , WB=0.24 (3-5:3) , SSI=0.65 (5-6:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

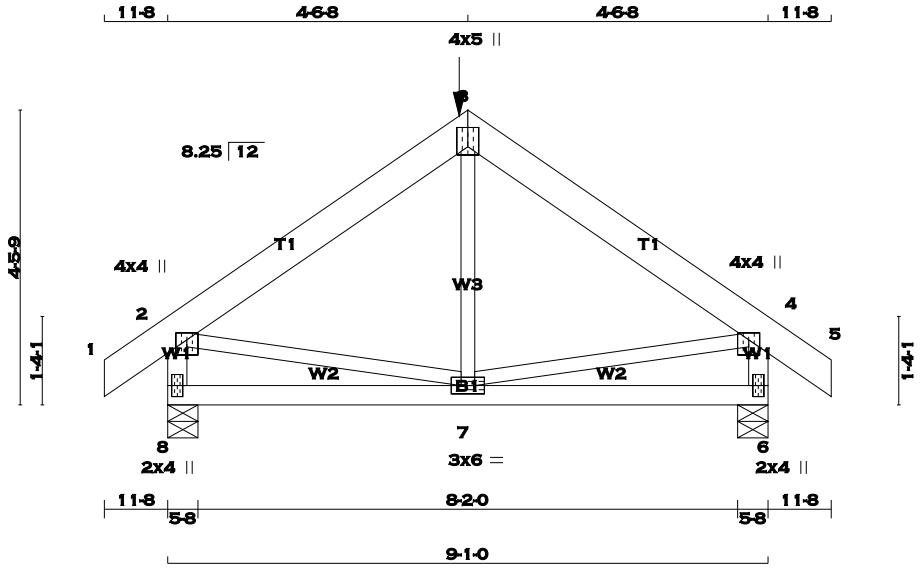
PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (3) (INPUT = 0.90)
JSI METAL= 0.28 (5) (INPUT = 1.00)

CONTINUED ON PAGE 2



TOTAL WEIGHT = 2 X 47 = 94 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 3	2x6	DRY	No.2	SPF
3 - 5	2x6	DRY	No.2	SPF
8 - 2	2x4	DRY	No.2	SPF
6 - 4	2x4	DRY	No.2	SPF
8 - 6	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW+p	MT20	4.0	4.0	1.25	2.00
3	TTW+p	MT20	4.0	5.0	3.50	2.00
4	TMVW+p	MT20	4.0	4.0	1.25	2.00
6	BMV1+p	MT20	2.0	4.0		
7	BMWWW-t	MT20	3.0	6.0		
8	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 316.3 lbs FACTORED DOWN AND 223.2 lbs FACTORED UP AT 4-6-8 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
8	657	0	723	0	-320	5-8	5-8
6	657	0	723	-205	-320	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 8 FOR 320 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 6 FOR 320 LBS FACTORED UPLIFT

PROVIDE FOR 205 LBS FACTORED HORIZONTAL REACTION AT JOINT 6

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
8	459	335 / 0	0 / 0	0 / 0	164 / -309	124 / 0	0 / 0
6	459	335 / 0	0 / 0	0 / 0	164 / -309	124 / 0	0 / 0
HORIZONTAL REACTIONS							
6	---	0 / 0	0 / 0	0 / 0	146 / -146	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 8, 6

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 24	-77.3	-77.3	0.04 (1)	10.00	7- 3	-57 / 131	0.04 (11)
2- 3	-569 / 305	-77.3	-77.3	0.19 (7)	6.25	2- 7	-175 / 485	0.10 (4)
3- 4	-569 / 305	-77.3	-77.3	0.19 (8)	6.25	7- 4	-175 / 485	0.10 (3)
4- 5	0 / 24	-77.3	-77.3	0.04 (1)	10.00			
8- 2	-683 / 347	0.0	0.0	0.07 (1)	7.81			
6- 4	-683 / 347	0.0	0.0	0.07 (1)	7.81			
8- 7	-8 / 18	-22.3	-22.3	0.14 (11)	10.00			
7- 6	-187 / 196	-22.3	-22.3	0.14 (11)	6.25			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
3	4-6-8	-241	-316	223	FRONT	VERT	TOTAL

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	23.3	PSF
		DL =	3.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.0	PSF
TOTAL	LOAD	=	33.3	PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 4-6-8
END SETBACK = 3-6-11
END WALL WIDTH = 5-8
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.30")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.30")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.19 (2-3:7) , BC=0.14 (7-8:11) , WB=0.10 (2-7:4) , SSI=0.10 (2-3:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

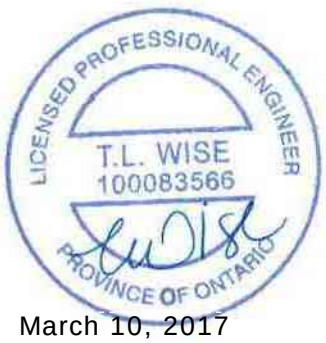
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

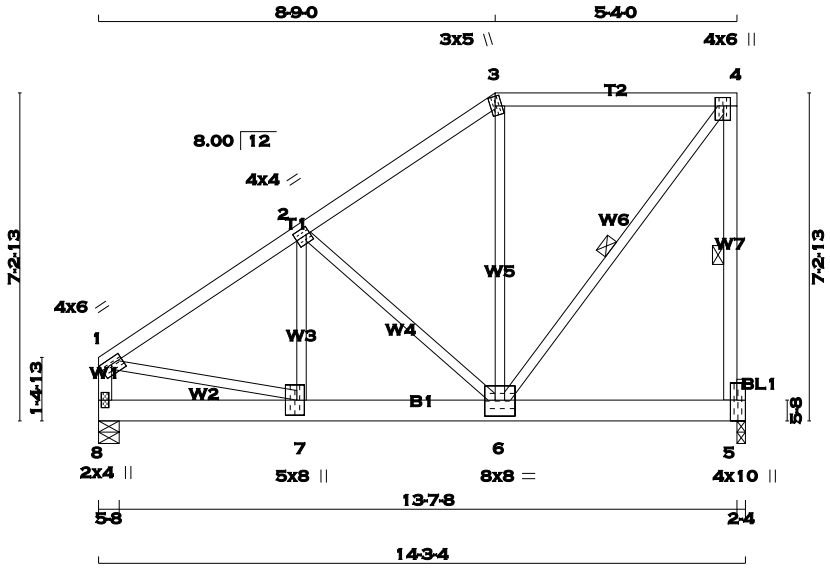
JSI GRIP= 0.90 (7) (INPUT = 0.90)
JSI METAL= 0.17 (2) (INPUT = 1.00)



March 10, 2017

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



SCALE = 1:50.8

TOTAL WEIGHT = 3 X 75 = 226 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 3	2x4	DRY	No.2	SPF
3 - 4	2x4	DRY	No.2	SPF
5 - 4	2x4	DRY	No.2	SPF
8 - 1	2x4	DRY	No.2	SPF
8 - 5	2x6	DRY	2100F 1.8E	SPF

BEARING BLOCKS

BL1	2x6	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS		
1- 3 1	12	TOP
3- 4 1	12	TOP
4- 5 1	12	TOP
8- 1 1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS		
8- 5 2	4	SIDE(671.2)
WEBS : (0.122"X3") SPIRAL NAILS		
2x3 1	6	

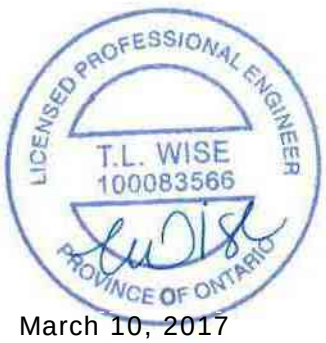
STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
1	TMVW-t	MT20	4.0	6.0	1.50	3.00
2	TMWW-t	MT20	4.0	4.0	2.00	1.00
3	TTW+m	MT20	3.0	5.0	2.50	1.25
4	TMVW+p	MT20	4.0	6.0	2.25	1.75
5	BMVK1+t	MT20	4.0	10.0	Edge	1.75
6	BMWWW-t	MT20	8.0	8.0	4.25	4.00



March 10, 2017

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
5	6652	0	7752	0	-2564	2-4	2-4
8	6641	0	7728	383	-2465	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 5 FOR 2564 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 8 FOR 2465 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 383 LBS FACTORED HORIZONTAL REACTION AT JOINT 8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
5	4668	3267 / 0	0 / 0	0 / 0	2749 / -2732	1401 / 0	0 / 0
8	4660	3260 / 0	0 / 0	0 / 0	2717 / -2661	1400 / 0	0 / 0
HORIZONTAL REACTIONS							
8	---	0 / 0	0 / 0	0 / 0	274 / -181	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 5, 8
BEARING SIZE FACTOR = 1.08 AT JNT(S) 5 (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.32 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x4 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 4-5. DBS = 6-0-0 . CBF = 182 LBS.
1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 4-6. DBS = 4-0-0 . CBF = 74 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) TO EACH PLY USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	FORCE (LBS)	MAX. FACTORED MAX CSI (LC)
1- 2	-7540 / 2432	-77.3	-77.3	0.19 (7)	4.32	7- 2	-966 / 3264	0.21 (3)
2- 3	-4764 / 1639	-77.3	-77.3	0.15 (7)	5.21	2- 6	-3215 / 1221	0.61 (3)
3- 4	-3991 / 1447	-77.3	-77.3	0.16 (7)	5.51	6- 3	-723 / 2293	0.20 (5)
5- 4	-5595 / 1937	0.0	0.0	0.36 (1)	4.96	6- 4	-2234 / 6775	0.44 (3)
8- 1	-5750 / 1887	0.0	0.0	0.17 (4)	6.13	1- 7	-1970 / 6423	0.42 (4)
8- 7	-364 / 245	-875.6	-875.6	0.20 (3)	6.25			
7- 6	-2107 / 6329	-875.6	-875.6	0.33 (3)	6.25			
6- 5	-169 / 176	-875.6	-875.6	0.24 (3)	6.25			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, [illegible] WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN} AND TRUSSES DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 38-4-8
END DISTANCE = 14-3-4
END SPAN CARRIED = 38-4-8
END WALL WIDTH = 2-4
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.46")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.46")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.36 (4-5:1) , BC=0.33 (6-7:3) , WB=0.61 (2-6:3) , SSI=0.53 (5-6:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (7) (INPUT = 0.90)
JSI METAL= 0.64 (1) (INPUT = 1.00)

CONTINUED ON PAGE 2

PLATES (table is in inches)

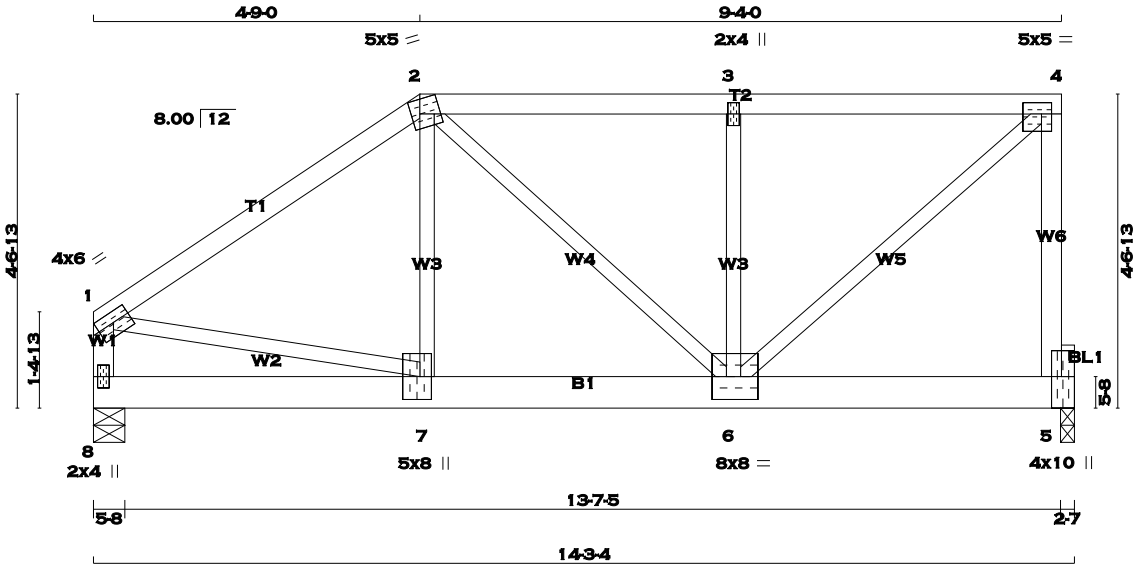
JT	TYPE	PLATES	W	LEN	Y	X
7	BMWW+t	MT20	5.0	8.0	4.00	2.00
8	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



TOTAL WEIGHT = 3 X 68 = 203 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 2	2x4	DRY	No.2	SPF
2 - 4	2x4	DRY	No.2	SPF
5 - 4	2x4	DRY	No.2	SPF
8 - 1	2x4	DRY	No.2	SPF
8 - 5	2x6	DRY	2100F 1.8E	SPF

BEARING BLOCKS

BL1	2x6	DRY	No.2	SPF

ALL WEBS EXCEPT | 2x3 | DRY | No.2 | SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)		LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
1- 2	1	12	TOP
2- 4	1	12	TOP
4- 5	1	12	TOP
8- 1	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
8- 5	2	5	SIDE(607.5)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

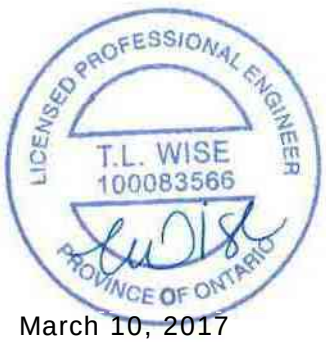
STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
1	TMVW-t	MT20	4.0	6.0	1.75	Edge
2	TTWW-m	MT20	5.0	5.0	2.50	1.50
3	TMW+w	MT20	2.0	4.0		
4	TMVW-t	MT20	5.0	5.0	2.00	1.75
5	BMVK1+t	MT20	4.0	10.0	Edge	1.75
6	BMWWW-t	MT20	8.0	8.0	4.00	2.50



March 10, 2017

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
5	6084	0	7062	0	-2328	2-7	2-7
8	6073	0	7058	233	-2290	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 5 FOR 2328 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 8 FOR 2290 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 233 LBS FACTORED HORIZONTAL REACTION AT JOINT 8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
5	4270	2988 / 0	0 / 0	0 / 0	2446 / -2486	1281 / 0	0 / 0
8	4262	2981 / 0	0 / 0	0 / 0	2464 / -2459	1281 / 0	0 / 0
HORIZONTAL REACTIONS							
8	---	0 / 0	0 / 0	0 / 0	167 / -117	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 5, 8
BEARING SIZE FACTOR = 1.09 AT JNT(S) 5 (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.46 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB.	FR-TO	FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
	1- 2	-6829 / 2229	-77.3	-77.3	0.21 (7)	4.46	7- 2	-1041 / 3579	0.23 (4)
	2- 3	-5607 / 1937	-77.3	-77.3	0.15 (7)	4.89	2- 6	-234 / 103	0.05 (3)
	3- 4	-5608 / 1938	-77.3	-77.3	0.15 (7)	4.88	6- 3	-423 / 348	0.04 (1)
	5- 4	-5254 / 1803	0.0	0.0	0.46 (1)	6.36	6- 4	-2554 / 7706	0.54 (7)
	8- 1	-5169 / 1743	0.0	0.0	0.15 (4)	6.40	1- 7	-1764 / 5767	0.38 (3)
	8- 7	-214 / 154	-794.1	-794.1	0.20 (3)	6.25			
	7- 6	-1847 / 5777	-794.1	-794.1	0.27 (3)	6.25			
	6- 5	-213 / 145	-794.1	-794.1	0.19 (3)	6.25			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 35-2-8
END DISTANCE = 14-3-4
END SPAN CARRIED = 35-2-8
END WALL WIDTH = 5-8
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.46")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.46")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.46 (4-5:1) , BC=0.27 (6-7:3) , WB=0.54 (4-6:7) , SSI=0.46 (7-8:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (2) (INPUT = 0.90)
JSI METAL= 0.58 (1) (INPUT = 1.00)

CONTINUED ON PAGE 2

PLATES (table is in inches)

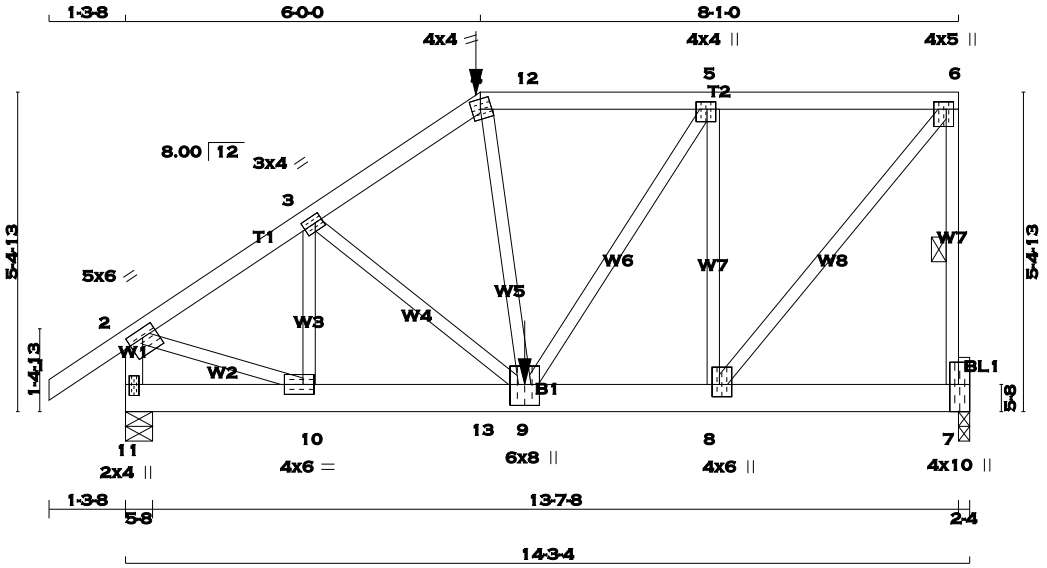
JT	TYPE	PLATES	W	LEN	Y	X
7	BMWW+t	MT20	5.0	8.0	4.00	2.00
8	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



SCALE = 1:38.9

TOTAL WEIGHT = 75 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 6	2x4	DRY	No.2	SPF
7 - 6	2x3	DRY	No.2	SPF
11 - 2	2x4	DRY	No.2	SPF
11 - 7	2x6	DRY	2100F 1.8E	SPF
BEARING BLOCKS				
BL1	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-t	MT20	5.0	6.0	1.75	3.00
3	TMWW-t	MT20	3.0	4.0	1.50	1.50
4	TTW-m	MT20	4.0	4.0	2.00	1.75
5	TMWW+t	MT20	4.0	4.0	1.50	1.75
6	TMVW+p	MT20	4.0	5.0	1.50	1.50
7	BMVK1+t	MT20	4.0	10.0	Edge	1.75
8	BMWW+t	MT20	4.0	6.0	2.50	1.50
9	BMWWW+t	MT20	6.0	8.0	4.25	3.00
10	BMWW-t	MT20	4.0	6.0	2.00	2.25
11	BMV1+p	MT20	2.0	4.0	2.25	1.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 455.0 lbs FACTORED DOWN AND 315.8 lbs FACTORED UP AT 6-0-0 ON TOP CHORD, AND 2059.8 lbs FACTORED DOWN AND 925.9 lbs FACTORED UP AT 6-9-0 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
7	1721	0	1968	0	-968	2-4	2-4
11	1979	0	2260	304	-1011	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 7 FOR 968 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 11 FOR 1011 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 304 LBS FACTORED HORIZONTAL REACTION AT JOINT 11

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
7	1206	852 / 0	0 / 0	0 / 0	617 / -919	354 / 0	0 / 0
11	1389	970 / 0	0 / 0	0 / 0	702 / -992	420 / 0	0 / 0
HORIZONTAL REACTIONS							
11	---	0 / 0	0 / 0	0 / 0	217 / -145	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 7, 11
BEARING SIZE FACTOR = 1.08 AT JNT(S) 7 (BASED ON SUPPORT DEPTH = 1-8)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.17 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 6-7. DBS = 8-0-0 . CBF = 84 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
1- 2	0 / 29	-77.3	-77.3	0.11 (1)	10.00
2- 3	-2404 / 1131	-77.3	-77.3	0.29 (7)	4.32
3- 4	-2585 / 1330	-77.3	-77.3	0.33 (7)	4.17
4-12	-2251 / 1189	-145.8	-145.8	0.34 (7)	4.32
12- 5	-2251 / 1189	-77.3	-77.3	0.34 (7)	4.32
5- 6	-1450 / 814	-77.3	-77.3	0.28 (7)	5.19
7- 6	-1937 / 991	0.0	0.0	0.53 (7)	4.20
11- 2	-2202 / 1030	0.0	0.0	0.21 (1)	5.75
11-10	-285 / 194	-33.0	-33.0	0.02 (11)	6.25
10-13	-961 / 2039	-33.0	-33.0	0.15 (3)	6.25
13- 9	-961 / 2039	-33.0	-33.0	0.15 (3)	6.25
9- 8	-616 / 1446	-17.5	-17.5	0.13 (3)	6.25
8- 7	-74 / 119	-17.5	-17.5	0.02 (11)	6.25

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
10- 3	-492 / 343	0.09 (3)
3- 9	-142 / 140	0.05 (5)
4- 9	-220 / 695	0.16 (1)
9- 5	-708 / 1507	0.41 (7)
8- 5	-1677 / 912	0.67 (3)
8- 6	-1156 / 2338	0.82 (7)
2-10	-858 / 2089	0.45 (1)

FACT... (LBS)

JT	...	...	...X+	FACE	DIR.	TYPE
4	...-0	...4	4	816	FRONT	VERT	TOTAL
9	...-0	1755	2060	926	FRONT	VERT	TOTAL

WIND ... PRESSURE (20) PSF AT
{40-... EXTERNAL PEAK
COE... STING SYSTEM; INTERNAL
WIN... MAY BE LOCATED ON
{OPE... LEAST 0-0-0 IN SX AWAY
FRO...

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP	CH.	LL =	23.3	PSF
		DL =	3.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.0	PSF
TOTAL	LOAD	=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 6-0-0
RIGHT SETBACK = 0-0
END SETBACK = 6-0-0
END WALL WIDTH = 5-8
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 6-9-0 OF SPAN MEASURED FROM THE LEFT.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.47")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.47")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.53 (6-7:7) , BC=0.15 (9-10:3) , WB=0.82 (6-8:7) , SSI=0.17 (5-6:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

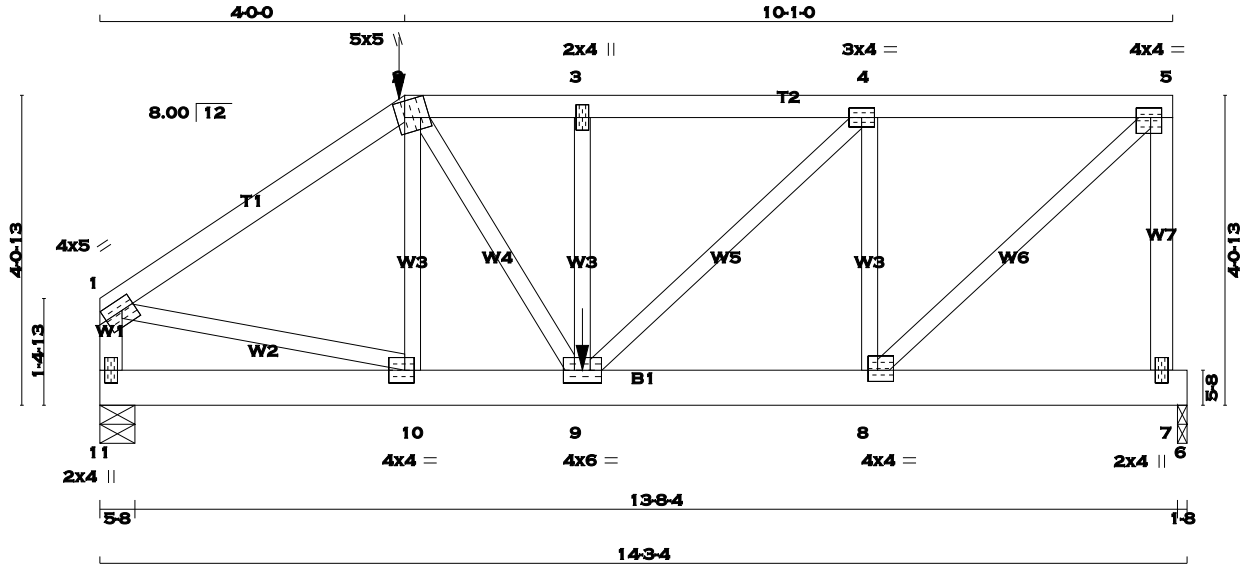
PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2

LICENSED PROFESSIONAL ENGINEER
T.L. WISE
100083566
PROVINCE OF ONTARIO
March 10, 2017

KOTT

RECEIVED
TOWN OF MILTON
MAR (20) 2017
17-1078
BUILDING DIVISION



TOTAL WEIGHT = 70 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 2	2x4	DRY	No.2	SPF
2 - 5	2x4	DRY	No.2	SPF
7 - 5	2x4	DRY	No.2	SPF
11 - 1	2x4	DRY	No.2	SPF
11 - 6	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
1	TMVW-t	MT20	4.0	5.0	1.75	Edge
2	TTWW+m	MT20	5.0	5.0	Edge	3.75
3	TMW+w	MT20	2.0	4.0		
4	TMWW-t	MT20	3.0	4.0		
5	TMVW-t	MT20	4.0	4.0	1.50	1.75
7	BMV+p	MT20	2.0	4.0		
8	BMWW-t	MT20	4.0	4.0	1.75	1.50
9	BMWWW-t	MT20	4.0	6.0		
10	BMWW-t	MT20	4.0	4.0	2.00	1.50
11	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 168.5 lbs FACTORED DOWN AND 117.0 lbs FACTORED UP AT 4-0-0 ON TOP CHORD, AND 570.1 lbs FACTORED DOWN AND 256.3 lbs FACTORED UP AT 6-4-0 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

March 10, 2017

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
11	1117	0	1252	206	-587	5-8	5-8
6	949	0	1042	0	-521	1-8	1-8

PROVIDE ANCHORAGE AT BEARING JOINT 11 FOR 587 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 6 FOR 521 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 206 LBS FACTORED HORIZONTAL REACTION AT JOINT 11

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
11	784	547 / 0	0 / 0	0 / 0	338 / -572	237 / 0	0 / 0
6	666	466 / 0	0 / 0	0 / 0	232 / -500	199 / 0	0 / 0
HORIZONTAL REACTIONS							
11	---	0 / 0	0 / 0	0 / 0	147 / -105	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 11, 6

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.24 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
1- 2	-1368 / 683	-77.3	-77.3	0.35 (7)	5.24
2- 3	-1419 / 784	-107.2	-107.2	0.18 (7)	5.40
3- 4	-1420 / 785	-77.3	-77.3	0.25 (7)	5.28
4- 5	-1012 / 581	-77.3	-77.3	0.22 (7)	6.01
7- 5	-1068 / 572	0.0	0.0	0.24 (1)	7.63
11- 1	-1208 / 616	0.0	0.0	0.12 (1)	7.29
11-10	-186 / 137	-24.2	-24.2	0.05 (11)	6.25
10- 9	-510 / 1144	-24.2	-24.2	0.19 (1)	6.25
9- 8	-436 / 1010	-17.5	-17.5	0.18 (1)	6.25
8- 7	-28 / 72	-17.5	-17.5	0.13 (1)	6.25
7- 6	0 / 0	-17.5	-17.5	0.13 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
10- 2	-137 / 156	0.03 (7)
9- 2	-274 / 510	0.12 (1)
9- 3	-292 / 226	0.07 (1)
9- 4	-285 / 570	0.13 (7)
8- 4	-764 / 481	0.18 (3)
8- 5	-694 / 1379	0.31 (1)
1-10	-470 / 1146	0.26 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
2	4-0-0	-129	-169	117	FRONT	VERT	TOTAL
9	6-4-0	-486	-570	256	FRONT	VERT	TOTAL

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM A

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP	CH.	LL	=	23.3 PSF
		DL	=	3.0 PSF
BOT	CH.	LL	=	0.0 PSF
		DL	=	7.0 PSF
TOTAL	LOAD		=	33.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 4-0-0
RIGHT SETBACK = 0-0
END SETBACK = 4-0-0
END WALL WIDTH = 5-8
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 6-4-0 OF SPAN MEASURED FROM THE LEFT.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.48")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.48")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.35 (1-2:7) , BC=0.19 (9-10:1) , WB=0.31 (5-8:1) , SSI=0.63 (6-7:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

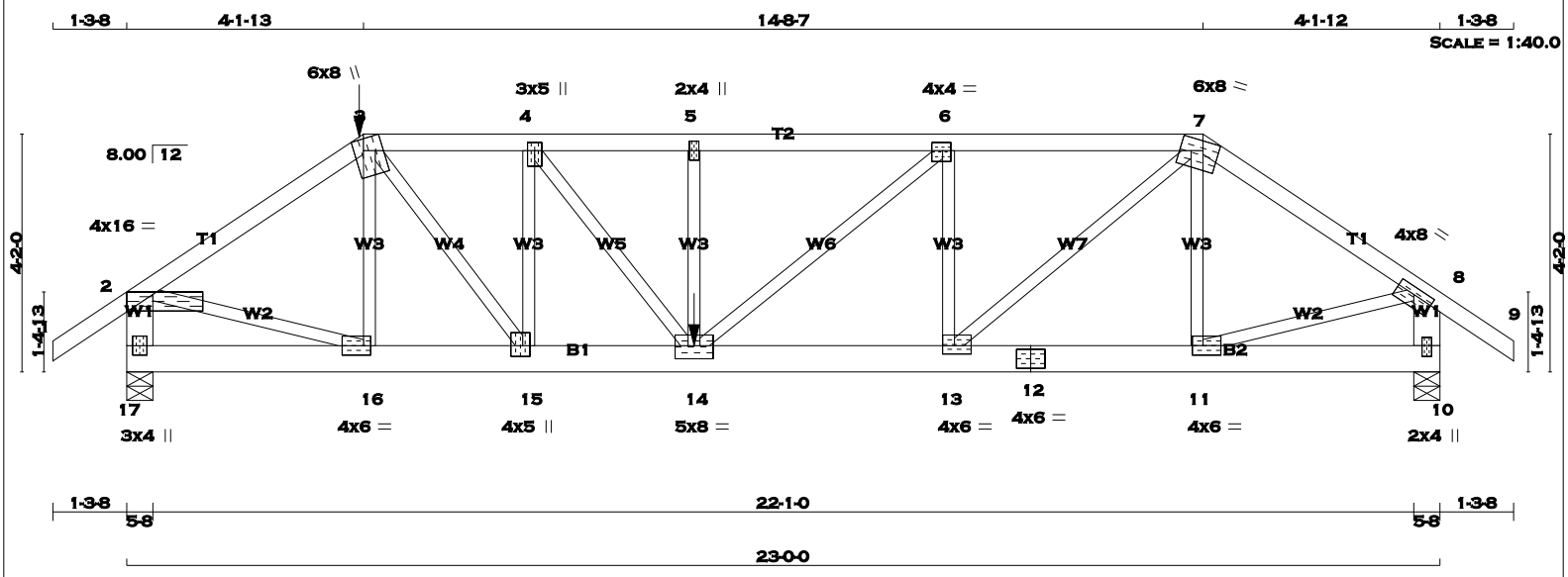
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2



TOTAL WEIGHT = 112 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 3	2x4	DRY	No.2	SPF
3 - 7	2x4	DRY	No.2	SPF
7 - 9	2x4	DRY	No.2	SPF
17 - 2	2x6	DRY	No.2	SPF
10 - 8	2x6	DRY	No.2	SPF
17 - 12	2x6	DRY	No.2	SPF
12 - 10	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

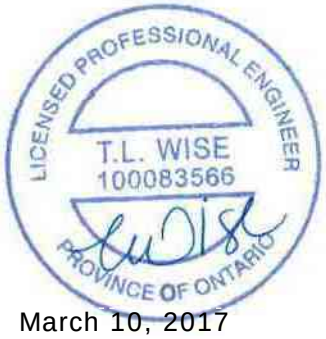
JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-p	MT20	4.0	16.0	Edge	5.50
3	TTWW+m	MT20	6.0	8.0	Edge	2.00
4	TMWW+t	MT20	3.0	5.0	1.75	1.50
5	TMW+w	MT20	2.0	4.0		
6	TMWW-t	MT20	4.0	4.0	1.75	1.75
7	TTWW-m	MT20	6.0	8.0	2.00	3.25
8	TMVW-t	MT20	4.0	8.0	1.50	3.75
10	BMV1+p	MT20	2.0	4.0	2.25	1.00
11	BMWW-t	MT20	4.0	6.0	2.00	2.25
12	BS-t	MT20	4.0	6.0		
13	BMWW-t	MT20	4.0	6.0	1.75	2.50
14	BMWWW-t	MT20	5.0	8.0	2.75	4.00
15	BMWW+t	MT20	4.0	5.0	2.25	1.50
16	BMWW-t	MT20	4.0	6.0	2.00	1.50
17	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 314.8 lbs FACTORED DOWN AND 218.5 lbs FACTORED UP AT 4-1-13 ON TOP CHORD, AND 1213.8 lbs FACTORED DOWN AND 545.6 lbs FACTORED UP AT 9-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.



March 10, 2017

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
17	2378	0	2694	203	-1273	5-8	5-8
10	1842	0	1995	0	-983	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 17 FOR 1273 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 10 FOR 983 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 203 LBS FACTORED HORIZONTAL REACTION AT JOINT 17

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
17	1668	1172 / 0	0 / 0	0 / 0	792 / -1228	495 / 0	0 / 0
10	1290	918 / 0	0 / 0	0 / 0	382 / -941	372 / 0	0 / 0
HORIZONTAL REACTIONS							
17	---	0 / 0	0 / 0	0 / 0	145 / -145	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 17, 10

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.06 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 5.95 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (11)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
1- 2	0 / 29	-77.3	-77.3	0.11 (1)		10.00
2- 3	-3115 / 1537	-77.3	-77.3	0.50 (7)		3.62
3- 4	-3729 / 1912	-145.8	-145.8	0.42 (7)		3.41
4- 5	-4345 / 2177	-145.8	-145.8	0.46 (7)		3.12
5- 6	-4345 / 2177	-77.3	-77.3	0.50 (7)		3.06
6- 7	-3419 / 1759	-77.3	-77.3	0.44 (8)		3.52
7- 8	-2270 / 1128	-77.3	-77.3	0.43 (8)		4.28
8- 9	0 / 29	-77.3	-77.3	0.11 (1)		10.00
17- 2	-2628 / 1311	0.0	0.0	0.17 (1)		6.44
10- 8	-1963 / 1004	0.0	0.0	0.13 (1)		7.20
17-16	-184 / 192	-33.0	-33.0	0.07 (11)		6.25
16-15	-1183 / 2598	-33.0	-33.0	0.31 (3)		6.25
15-14	-1732 / 3738	-33.0	-33.0	0.53 (3)		5.95
14-13	-1551 / 3428	-17.5	-17.5	0.50 (1)		6.20
13-12	-774 / 1884	-17.5	-17.5	0.24 (1)		6.25
12-11	-774 / 1884	-17.5	-17.5	0.24 (1)		6.25
11-10	-9 / 20	-17.5	-17.5	0.04 (11)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
16- 3	-472 / 372	0.11 (3)
3-15	-950 / 1874	0.41 (1)
15- 4	-1347 / 780	0.32 (3)
4-14	-481 / 1016	0.23 (1)
14- 5	-390 / 294	0.09 (3)
14- 6	-585 / 1217	0.33 (7)
13- 6	-1195 / 696	0.29 (3)
13- 7	-1018 / 2023	0.55 (7)
11- 7	-360 / 246	0.09 (3)
2-16	-1172 / 2631	0.57 (1)
11- 8	-822 / 1945	0.43 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
3	4-1-13	-241	-315	219	FRONT	VERT	TOTAL
14	9-11-4	-1034	-1214	546	FRONT	VERT	TOTAL

WIND LOADS APPLIED AS PRESSURE FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0 FROM LEFT END} MEAN HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, {MAIN WIND FORCE RESISTING SYSTEM} INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST 20.0 FEET FROM

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

KOTT

RECEIVED
TOWN OF MILTON
MAR 29, 2017
17-4978
BUILDING DIVISION

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP	CH.	LL =	23.3	PSF
		DL =	3.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.0	PSF
TOTAL	LOAD	=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 4-1-13
RIGHT SETBACK = 4-1-12
END SETBACK = 6-0-0
END WALL WIDTH = 5-8
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 9-11-4 OF SPAN MEASURED FROM THE LEFT.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.77")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.77")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.50 (5-6:7) , BC=0.53 (14-15:3) , WB=0.57 (2-16:1) , SSI=0.22 (3-4:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

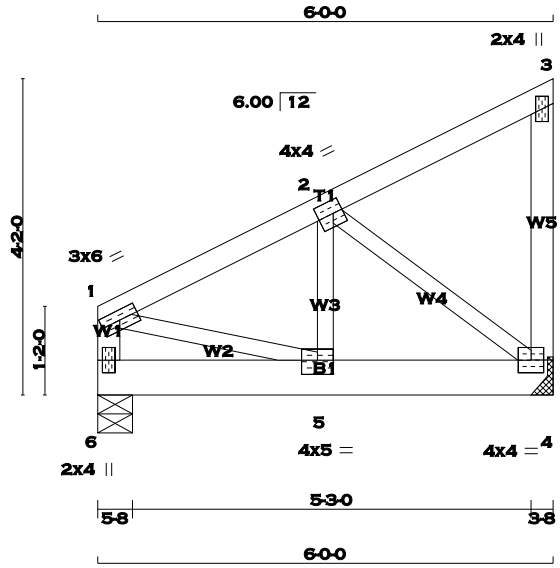
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2



SCALE = 1:30.4

TOTAL WEIGHT = 29 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 3	2x4	DRY	No.2	SPF
4 - 3	2x4	DRY	No.2	SPF
6 - 1	2x4	DRY	No.2	SPF
6 - 4	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
1	TMVW-t	MT20	3.0	6.0		
2	TMWW-t	MT20	4.0	4.0	2.00	1.50
3	TMV+p	MT20	2.0	4.0		
4	BMVW1-t	MT20	4.0	4.0		
5	BMWW-t	MT20	4.0	5.0	2.25	2.50
6	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
4	1034	0	1187	0	-454	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	
6	1034	0	1186	242	-389	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 4 FOR 454 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 6 FOR 389 LBS FACTORED UPLIFT

PROVIDE FOR 242 LBS FACTORED HORIZONTAL REACTION AT JOINT 6

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
4	726	508 / 0	0 / 0	0 / 0	381 / -465	218 / 0	0 / 0
6	726	508 / 0	0 / 0	0 / 0	379 / -418	218 / 0	0 / 0
HORIZONTAL REACTIONS							
6	---	0 / 0	0 / 0	0 / 0	173 / -70	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 6

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.10 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	-1022 / 337	-77.3	-77.3	0.16 (7)	6.10	5- 2	-231 / 855	0.18 (3)
2- 3	-72 / 71	-77.3	-77.3	0.11 (7)	6.25	2- 4	-1147 / 491	0.26 (3)
4- 3	-100 / 78	0.0	0.0	0.10 (7)	7.81	1- 5	-258 / 955	0.21 (4)
6- 1	-792 / 287	0.0	0.0	0.08 (1)	7.81			
6- 5	-226 / 91	-267.5	-267.5	0.16 (3)	6.25			
5- 4	-369 / 915	-267.5	-267.5	0.26 (3)	6.25			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 13-0-0
END DISTANCE = 6-0-0
END SPAN CARRIED = 13-0-0
END WALL WIDTH = 5-8
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.16 (1-2:7) , BC=0.26 (4-5:3) , WB=0.26 (2-4:3) , SSI=0.31 (5-6:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (4) (INPUT = 0.90)
JSI METAL= 0.34 (4) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
T.L. WISE
100083566
PROVINCE OF ONTARIO
March 10, 2017

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

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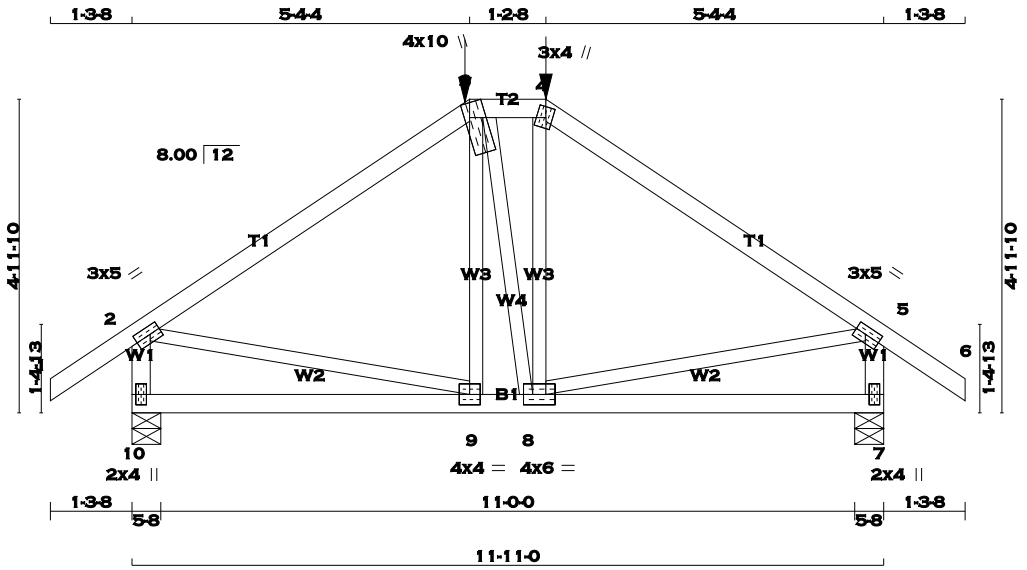

TOTAL WEIGHT = 55 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 3	2x4	DRY	No.2	SPF
3 - 4	2x4	DRY	No.2	SPF
4 - 6	2x4	DRY	No.2	SPF
10 - 2	2x4	DRY	No.2	SPF
7 - 5	2x4	DRY	No.2	SPF
10 - 7	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-t	MT20	3.0	5.0	1.50	2.00
3	TTWW+m	MT20	4.0	10.0	Edge	1.00
4	TTW+m	MT20	3.0	4.0	2.00	1.25
5	TMVW-t	MT20	3.0	5.0	1.50	2.00
7	BMV1+p	MT20	2.0	4.0		
8	BMWWW-t	MT20	4.0	6.0		
9	BMWW-t	MT20	4.0	4.0		
10	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 225.5 lbs FACTORED DOWN AND 156.6 lbs FACTORED UP AT 6-6-12, AND 225.5 lbs FACTORED DOWN AND 156.6 lbs FACTORED UP AT 5-4-4 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
10	902	0	999	-239	-450	5-8	5-8
7	902	0	996	0	-450	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 10 FOR 450 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 7 FOR 450 LBS FACTORED UPLIFT

PROVIDE FOR 239 LBS FACTORED HORIZONTAL REACTION AT JOINT 10

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
10	630	456 / 0	0 / 0	0 / 0	243 / -434	174 / 0	0 / 0
7	630	456 / 0	0 / 0	0 / 0	235 / -434	174 / 0	0 / 0
HORIZONTAL REACTIONS							
10	---	0 / 0	0 / 0	0 / 0	171 / -171	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 10, 7

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.94 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF)	CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 29	-77.3	-77.3	0.11 (1)		10.00	9- 3	-52 / 104	0.02 (11)
2- 3	-851 / 447	-77.3	-77.3	0.50 (7)		5.94	3- 8	-76 / 93	0.03 (5)
3- 4	-711 / 454	-107.2	-107.2	0.09 (8)		6.25	8- 4	-65 / 165	0.04 (5)
4- 5	-858 / 448	-77.3	-77.3	0.50 (8)		5.96	2- 9	-257 / 730	0.16 (4)
5- 6	0 / 29	-77.3	-77.3	0.11 (1)		10.00	8- 5	-262 / 735	0.16 (3)
10- 2	-945 / 488	0.0	0.0	0.10 (1)		7.81			
7- 5	-940 / 488	0.0	0.0	0.10 (1)		7.81			
10- 9	-218 / 229	-24.2	-24.2	0.18 (11)		6.25			
9- 8	-285 / 724	-24.2	-24.2	0.24 (11)		6.25			
8- 7	-10 / 21	-24.2	-24.2	0.19 (11)		10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
3	5-4-4	-173	-226	157	FRONT	VERT	TOTAL
4	6-6-12	-173	-226	157	FRONT	VERT	TOTAL

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	23.3	PSF
		DL =	3.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.0	PSF
TOTAL	LOAD	=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 5-4-4
END SETBACK = 4-0-0
END WALL WIDTH = 5-8
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.40")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.40")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.50 (4-5:8) , BC=0.24 (8-9:11) , WB=0.16 (5-8:3) , SSI=0.15 (2-3:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (8) (INPUT = 0.90)
JSI METAL= 0.31 (5) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
T.L. WISE
100083566
PROVINCE OF ONTARIO
March 10, 2017

KOTT

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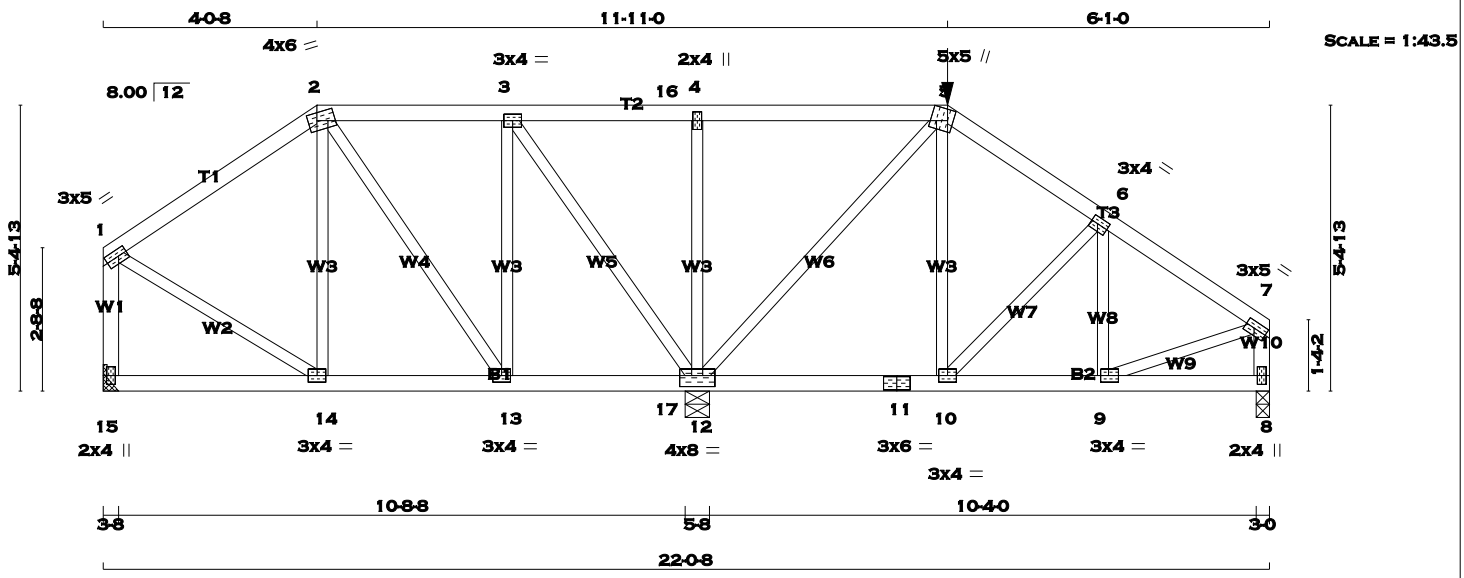


TOTAL WEIGHT = 99 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 2	2x4	DRY	No.2	SPF
2 - 5	2x4	DRY	No.2	SPF
5 - 7	2x4	DRY	No.2	SPF
15 - 1	2x4	DRY	No.2	SPF
8 - 7	2x4	DRY	No.2	SPF
15 - 11	2x4	DRY	No.2	SPF
11 - 8	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

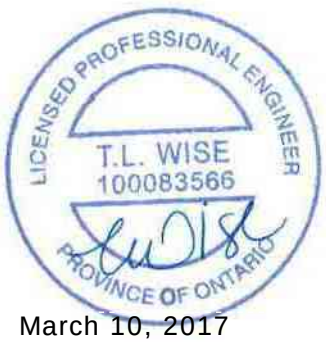
PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
1	TMVW-t	MT20	3.0	5.0	1.50	2.00
2	TTWW-m	MT20	4.0	6.0	1.75	2.00
3	TMWW-t	MT20	3.0	4.0		
4	TMW+w	MT20	2.0	4.0		
5	TTWW+m	MT20	5.0	5.0	Edge	3.75
6	TMWW-t	MT20	3.0	4.0	1.50	1.50
7	TMVW-t	MT20	3.0	5.0	1.50	2.00
8	BMV1+p	MT20	2.0	4.0		
9	BMWW-t	MT20	3.0	4.0	1.50	1.75
10, 13, 14						
10	BMWW-t	MT20	3.0	4.0		
11	BS-t	MT20	3.0	6.0		
12	BMWWW1-t	MT20	4.0	8.0	2.50	4.00
15	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 461.3 lbs FACTORED DOWN AND 320.2 lbs FACTORED UP AT 15-11-8 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.



March 10, 2017

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
15	454	0	480	-248	-212	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	
12	1820	0	2041	0	-1039	5-8	5-8
8	747	0	850	0	-341	3-0	3-0

PROVIDE ANCHORAGE AT BEARING JOINT 15 FOR 212 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 12 FOR 1039 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 8 FOR 341 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 248 LBS FACTORED HORIZONTAL REACTION AT JOINT 15

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
15	319	219 / 0	0 / 0	0 / 0	65 / -216	100 / 0	0 / 0
12	1274	909 / 0	0 / 0	0 / 0	552 / -977	365 / 0	0 / 0
8	529	345 / 0	0 / 0	0 / 0	257 / -362	184 / 0	0 / 0
HORIZONTAL REACTIONS							
15	---	0 / 0	0 / 0	0 / 0	154 / -177	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 8

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	-283 / 180	-77.3	-77.3	0.28 (7)	6.25	14- 2	-42 / 71	0.02 (1)
2- 3	-165 / 195	-77.3	-77.3	0.15 (1)	6.25	2-13	-145 / 79	0.10 (3)
3-16	-4 / 107	-77.3	-77.3	0.45 (3)	10.00	13- 3	-27 / 153	0.03 (3)
16- 4	-4 / 107	-149.9	-149.9	0.45 (3)	10.00	3-12	-473 / 260	0.32 (3)
4- 5	-5 / 108	-149.9	-149.9	0.46 (3)	10.00	12- 4	-769 / 561	0.33 (3)
5- 6	-732 / 456	-77.3	-77.3	0.20 (8)	6.25	12- 5	-1037 / 547	0.91 (4)
6- 7	-833 / 389	-77.3	-77.3	0.19 (8)	6.25	10- 5	-15 / 266	0.08 (11)
15- 1	-451 / 232	0.0	0.0	0.07 (7)	7.81	10- 6	-195 / 165	0.06 (4)
8- 7	-803 / 372	0.0	0.0	0.08 (1)	7.81	9- 6	-117 / 157	0.03 (8)
						1-14	-30 / 249	0.06 (1)
15-14	-167 / 226	-17.5	-17.5	0.08 (11)	6.25	9- 7	-260 / 723	0.16 (1)
14-13	-130 / 251	-17.5	-17.5	0.10 (11)	6.25			
13-17	-89 / 201	-17.5	-17.5	0.15 (11)	6.25			
17-12	-89 / 201	-33.9	-33.9	0.15 (11)	6.25			
12-11	-203 / 580	-33.9	-33.9	0.19 (1)	6.25			
11-10	-203 / 580	-33.9	-33.9	0.19 (1)	6.25			
10- 9	-227 / 679	-33.9	-33.9	0.17 (11)	6.25			
9- 8	-9 / 20	-33.9	-33.9	0.08 (11)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	OC	C	+	ACE	DIR.	TYPE
5	5-	4	320	FRONT	VERT	TOTAL

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF {0.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXPOSURE {B} CLOSED BUILDING COE STING SYSTEM}.INTERNAL WIN C MAY BE {0.2} OR {-0.3}. {OPE LEAST {0-0} FT-IN-SX AWAY FRO

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

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BUILDING DIVISION

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 4-0-8
RIGHT SETBACK = 6-1-0
END SETBACK = 6-0-0
END WALL WIDTH = 3-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 11-5-8 OF SPAN MEASURED FROM THE RIGHT.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.36")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.36")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.46 (4-5:3) , BC=0.19 (10-12:1) , WB=0.91 (5-12:4) , SSI=0.36 (4-5:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

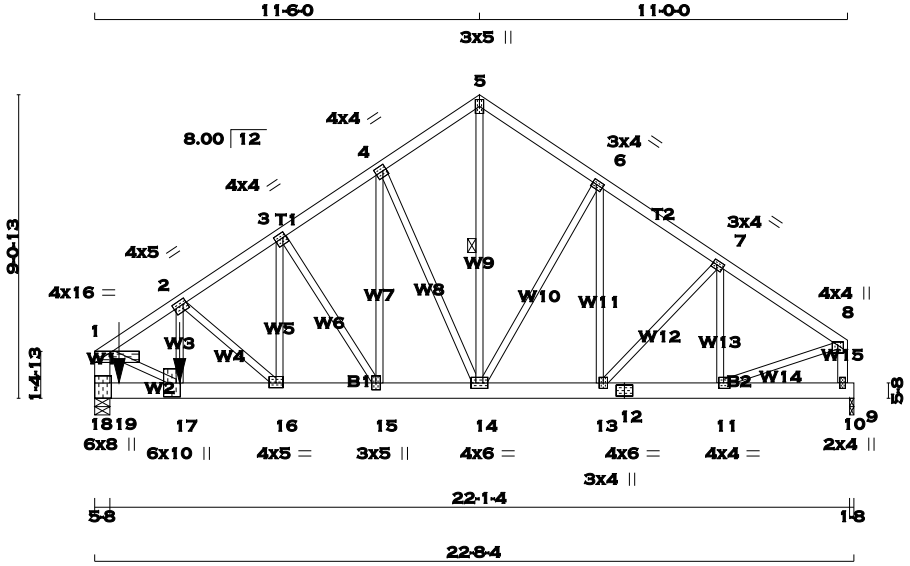
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (9) (INPUT = 0.90)
JSI METAL= 0.27 (7) (INPUT = 1.00)



TOTAL WEIGHT = 2 X 127 = 254 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 5	2x4	DRY	No.2	SPF
5 - 8	2x4	DRY	No.2	SPF
18 - 1	2x6	DRY	No.2	SPF
10 - 8	2x4	DRY	No.2	SPF
18 - 12	2x6	DRY	No.2	SPF
12 - 9	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)		LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS				
1- 5	1	12		TOP
5- 8	1	12		TOP
10- 8	1	12		TOP
18- 1	2	12		TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS				
18- 12	2	12		SIDE(0.0)
12- 9	2	12		TOP
WEBS : (0.122"X3") SPIRAL NAILS				
2x3	1	6		

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

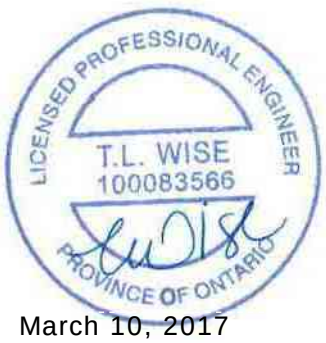
GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
1	TMVW-p	MT20	4.0	16.0	0.50	Edge
2	TMWW-t	MT20	4.0	5.0	1.75	1.25
3	TMWW-t	MT20	4.0	4.0	1.75	1.00
4	TMWW-t	MT20	4.0	4.0	2.00	1.25
5	TTW+p	MT20	3.0	5.0		



March 10, 2017

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
18	6489	0	7496	365	-3307	5-8	5-8
9	1655	0	1800	0	-750	1-8	1-8

PROVIDE ANCHORAGE AT BEARING JOINT 18 FOR 3307 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 9 FOR 750 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 365 LBS FACTORED HORIZONTAL REACTION AT JOINT 18

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
18	4554	3186 / 0	0 / 0	0 / 0	2519 / -3242	1368 / 0	0 / 0
9	1161	811 / 0	0 / 0	0 / 0	365 / -761	351 / 0	0 / 0
HORIZONTAL REACTIONS							
18	---	0 / 0	0 / 0	0 / 0	261 / -255	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 18, 9

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.60 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 5-14. DBS = 16-0-0 . CBF = 90 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) TO EACH PLY USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
1- 2	-7210 / 3199	-77.3	-77.3	0.29 (7)	3.60	
2- 3	-4296 / 1936	-77.3	-77.3	0.19 (7)	4.58	
3- 4	-2868 / 1354	-77.3	-77.3	0.15 (7)	5.41	
4- 5	-2013 / 1061	-77.3	-77.3	0.13 (7)	6.18	
5- 6	-2026 / 1056	-77.3	-77.3	0.15 (8)	6.16	
6- 7	-2127 / 999	-77.3	-77.3	0.16 (8)	6.04	
7- 8	-2053 / 887	-77.3	-77.3	0.15 (8)	6.12	
18- 1	-6464 / 2863	0.0	0.0	0.20 (1)	5.92	
10- 8	-1869 / 813	0.0	0.0	0.10 (1)	7.81	
18-19	-346 / 348	-17.5	-17.5	0.24 (3)	6.25	
19-17	-346 / 348	-17.5	-17.5	0.24 (3)	6.25	
17-16	-2770 / 6068	-17.5	-17.5	0.48 (3)	6.25	
16-15	-1597 / 3649	-17.5	-17.5	0.24 (3)	6.25	
15-14	-971 / 2422	-17.5	-17.5	0.18 (1)	6.25	
14-13	-61[illegible] / 1[illegible]	[illegible]	[illegible]	[illegible]	6.25	
13-12	[illegible] / 1[illegible]	17	[illegible]5	0.1[illegible] (1)	6.25	
12-11	[illegible] / 1[illegible]	17	[illegible]5	0.1[illegible] (1)	6.25	
11-10	[illegible] / 2[illegible]	17	[illegible]5	0.1[illegible] (1)	6.25	
10- 9	0/ 0	[illegible]	-17.5	0.1[illegible] (1)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
17- 2	-1603 / 3590	0.39 (3)
2-16	-3257 / 1579	0.43 (3)
16- 3	-1109 / 2427	0.26 (3)
3-15	-2283 / 1164	0.57 (3)
15- 4	-884 / 1855	0.36 (7)
4-14	-1813 / 1003	0.92 (3)
14- 5	-1031 / 2000	0.26 (7)
14- 6	-274 / 310	0.13 (4)
13- 6	-76 / 138	0.03 (3)
13- 7	-107 / 137	0.02 (6)
11- 7	-398 / 225	0.05 (3)
1-17	-2780 / 6417	0.69 (3)
11- 8	-681 / 1823	0.21 (1)

FACT
JT
17 TOTAL
19 TOTAL

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP	CH.	LL	=	23.3 PSF
		DL	=	3.0 PSF
BOT	CH.	LL	=	0.0 PSF
		DL	=	7.0 PSF
TOTAL	LOAD		=	33.3 PSF

SPACING = 24.0 IN. C/C

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.76")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.76")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.29 (1-2:7) , BC=0.48 (16-17:3) , WB=0.92 (4-14:3) , SSI=0.55 (9-10:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (7) (INPUT = 0.90)
JSI METAL= 0.65 (17) (INPUT = 1.00)

CONTINUED ON PAGE 2

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
6	TMWW-t	MT20	3.0	4.0	1.50	1.50
7	TMWW-t	MT20	3.0	4.0	1.50	1.50
8	TMVW+p	MT20	4.0	4.0	1.25	2.00
10	BMV+p	MT20	2.0	4.0		
11	BMWW-t	MT20	4.0	4.0	2.00	1.75
12	BS-t	MT20	4.0	6.0		
13	BMWW+t	MT20	3.0	4.0		
14	BMWWW-t	MT20	4.0	6.0		
15	BMWW+t	MT20	3.0	5.0		
16	BMWW-t	MT20	4.0	5.0	1.75	2.50
17	BMWW+t	MT20	6.0	10.0	5.00	1.50
18	BMV1+t	MT20	6.0	8.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

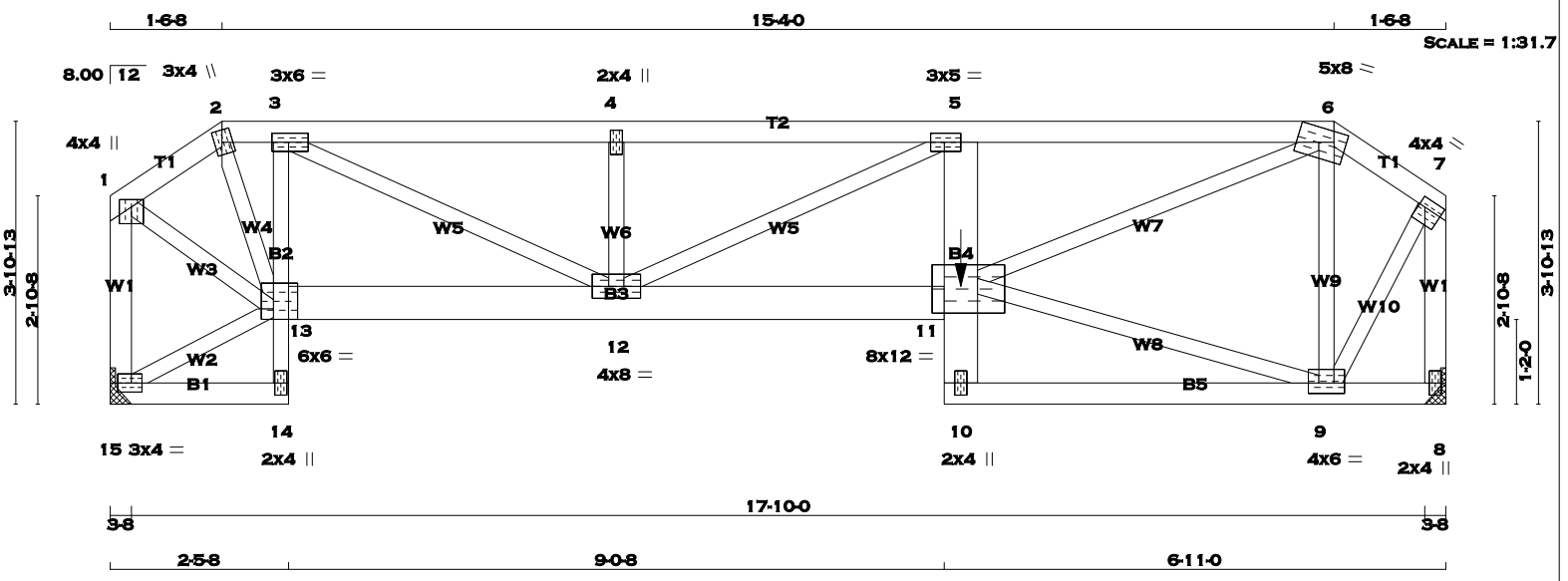
HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1172.0 lbs FACTORED DOWN AND 526.8 lbs FACTORED UP AT 8-12, AND 5875.2 lbs FACTORED DOWN AND 2640.9 lbs FACTORED UP AT 2-6-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



TOTAL WEIGHT = 91 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 2	2x4	DRY	No.2	SPF
2 - 6	2x4	DRY	No.2	SPF
6 - 7	2x4	DRY	No.2	SPF
15 - 1	2x4	DRY	No.2	SPF
8 - 7	2x4	DRY	No.2	SPF
15 - 14	2x4	DRY	No.2	SPF
14 - 3	2x3	DRY	No.2	SPF
13 - 11	2x6	DRY	No.2	SPF
10 - 5	2x6	DRY	No.2	SPF
10 - 8	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

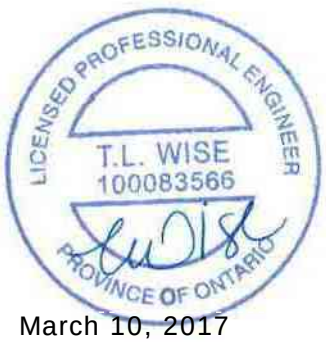
JT	TYPE	PLATES	W	LEN	Y	X
1	TMVW+p	MT20	4.0	4.0	1.25	2.00
2	TTW+m	MT20	3.0	4.0	2.00	1.25
3	TMVW-t	MT20	3.0	6.0	1.50	2.75
4	TMW+w	MT20	2.0	4.0		
5	TMVW-t	MT20	3.0	5.0	1.50	2.25
6	TTWW-m	MT20	5.0	8.0	1.75	2.00
7	TMVW-t	MT20	4.0	4.0	1.50	1.00
8	BMV1+p	MT20	2.0	4.0		
9	BMWWW-t	MT20	4.0	6.0	1.75	3.00
10	BMV+p	MT20	2.0	4.0		
11	BVMWW-l	MT20	8.0	12.0	5.75	7.50
12	BMWWW-t	MT20	4.0	8.0		
13	BVMWWW-l	MT20	6.0	6.0	Edge	4.00
14	BMV+p	MT20	2.0	4.0		
15	BMVW1-t	MT20	3.0	4.0	1.50	1.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 260.8 lbs FACTORED DOWN AND 117.2 lbs FACTORED UP AT 12-2-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.



March 10, 2017

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
8	1015	0	1075	0	-529	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	
15	954	0	1010	-188	-497	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

PROVIDE ANCHORAGE AT BEARING JOINT 8 FOR 529 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 15 FOR 497 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 188 LBS FACTORED HORIZONTAL REACTION AT JOINT 15

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
8	712	498 / 0	0 / 0	0 / 0	150 / -515	214 / 0	0 / 0
15	670	468 / 0	0 / 0	0 / 0	139 / -484	201 / 0	0 / 0
HORIZONTAL REACTIONS							
15	---	0 / 0	0 / 0	0 / 0	134 / -134	0 / 0	0 /0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.17 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	-933 / 488	-77.3	-77.3	0.11 (7)	6.25	3-12	-572 / 1166	0.27 (1)
2- 3	-966 / 544	-77.3	-77.3	0.20 (7)	6.12	12- 4	-372 / 285	0.06 (1)
3- 4	-2011 / 1033	-77.3	-77.3	0.29 (7)	4.58	12- 5	-564 / 309	0.25 (3)
4- 5	-2011 / 1033	-77.3	-77.3	0.35 (7)	4.49	11- 9	-63 / 226	0.05 (1)
5- 6	-2347 / 1208	-77.3	-77.3	0.38 (1)	4.17	11- 6	-1008 / 2050	0.48 (7)
6- 7	-558 / 329	-77.3	-77.3	0.09 (8)	6.25	9- 6	-687 / 362	0.16 (1)
15- 1	-1003 / 498	0.0	0.0	0.15 (1)	7.79	9- 7	-346 / 854	0.20 (1)
8- 7	-1090 / 522	0.0	0.0	0.17 (1)	7.58	1-13	-364 / 853	0.20 (1)
						15-13	-152 / 185	0.04 (5)
15-14	-3 / 7	-17.5	-17.5	0.03 (11)	10.00	2-13	-321 / 583	0.13 (1)
14-13	0 / 23	0.0	0.0	0.03 (1)	10.00			
13- 3	-879 / 534	0.0	0.0	0.12 (8)	7.21			
13-12	-421 / 973	-17.5	-17.5	0.17 (1)	6.25			
12-11	-1147 / 2523	-17.5	-17.5	0.36 (1)	6.25			
10-11	0 / 40	0.0	0.0	0.24 (1)	10.00			
11- 5	-190 / 218	0.0	0.0	0.24 (1)	7.81			
10- 9	-107 / 227	-17.5	-17.5	0.13 (11)	6.25			
9- 8	-23 / 50	-17.5	-17.5	0.11 (11)	6.25			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
11	12-2-4	-222	-261	117	FRONT	VERT	TOTAL

WIND LOAD APPLIED TO THE TRUSS WAS DETERMINED FROM A REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0 FT} MEAN ROOF HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, AND BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM} INTERNAL WIND PRESSURE WAS ASSUMED IN DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0.0 FT} IN SX AWAY FROM EAVE.

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

KOTT

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP	CH.	LL =	23.3	PSF
		DL =	3.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.0	PSF
TOTAL	LOAD	=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.38 (5-6:1) , BC=0.36 (11-12:1) , WB=0.48 (6-11:7) , SSI=0.20 (5-6:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

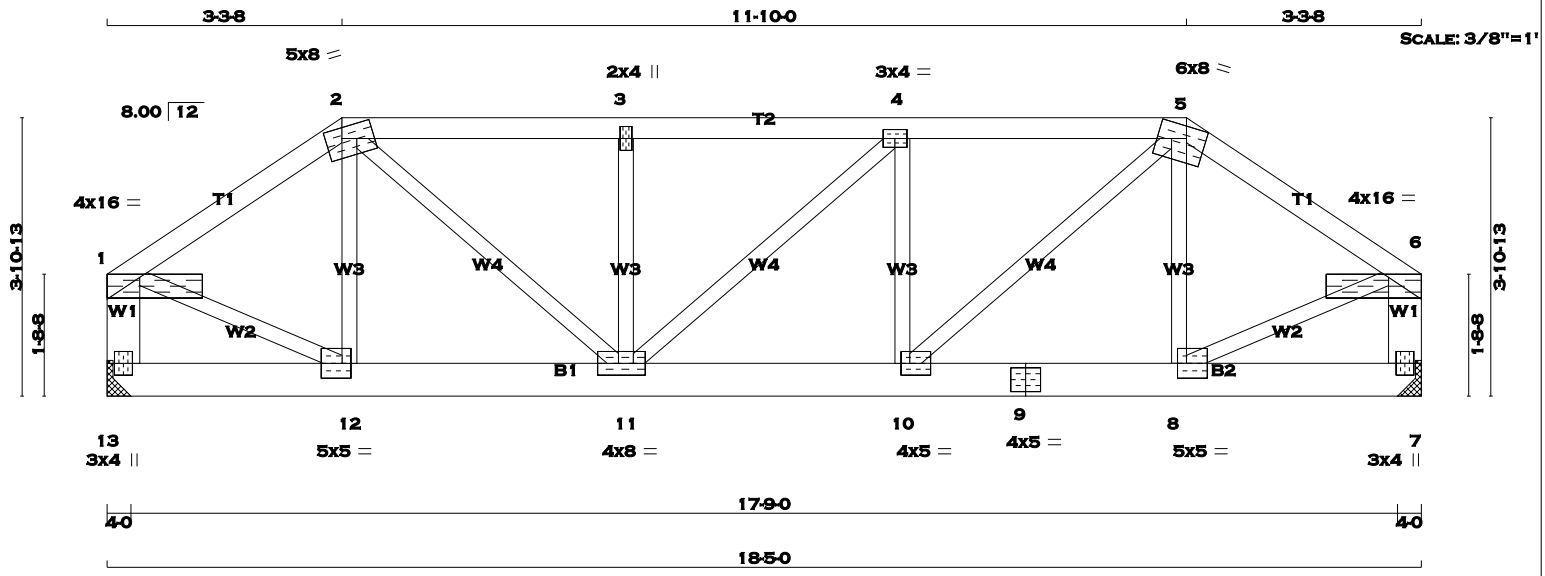
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (6) (INPUT = 0.90)
JSI METAL= 0.40 (6) (INPUT = 1.00)



TOTAL WEIGHT = 2 X 87 = 175 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 2	2x4	DRY	No.2	SPF
2 - 5	2x4	DRY	No.2	SPF
5 - 6	2x4	DRY	No.2	SPF
13 - 1	2x6	DRY	No.2	SPF
7 - 6	2x6	DRY	No.2	SPF
13 - 9	2x6	DRY	No.2	SPF
9 - 7	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
1- 2	1	12	TOP
2- 5	1	12	TOP
5- 6	1	12	TOP
13- 1	2	12	TOP
7- 6	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
13- 9	2	12	SIDE(248.2)
9- 7	2	12	SIDE(248.2)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

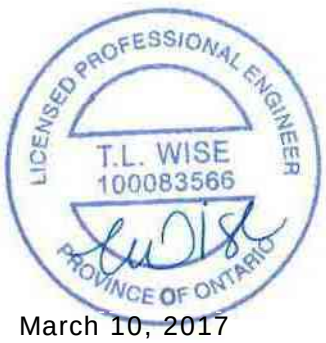
NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
1	TMVW-p	MT20	4.0	16.0	0.50	Edge
2	TTWW-m	MT20	5.0	8.0	1.75	2.75
3	TMW+w	MT20	2.0	4.0		
4	TMWW-t	MT20	3.0	4.0		
5	TTWW-m	MT20	6.0	8.0	2.00	3.25
6	TMVW-p	MT20	4.0	16.0	Edge	5.50



March 10, 2017

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
13	4770	0	5496	-162	-1855	HANGER BY OTHERS MIN. SEAT SIZE: 4-0	
7	4770	0	5474	0	-1855	HANGER BY OTHERS MIN. SEAT SIZE: 4-0	

PROVIDE ANCHORAGE AT BEARING JOINT 13 FOR 1855 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 7 FOR 1855 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 162 LBS FACTORED HORIZONTAL REACTION AT JOINT 13

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
13	3347	2342 / 0	0 / 0	0 / 0	1816 / -1972	1006 / 0	0 / 0
7	3347	2342 / 0	0 / 0	0 / 0	1761 / -1972	1006 / 0	0 / 0
HORIZONTAL REACTIONS							
13	---	0 / 0	0 / 0	0 / 0	115 / -115	0 / 0	0 /0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.63 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	-5339 / 1832	-77.3	-77.3	0.22 (7)	4.14	12- 2	-124 / 428	0.05 (4)
2- 3	-6995 / 2463	-77.3	-77.3	0.25 (7)	3.63	2-11	-1199 / 3384	0.37 (1)
3- 4	-6995 / 2464	-77.3	-77.3	0.24 (7)	3.67	11- 3	-308 / 254	0.03 (1)
4- 5	-7004 / 2472	-77.3	-77.3	0.25 (8)	3.63	11- 4	-34 / 45	0.01 (4)
5- 6	-5321 / 1830	-77.3	-77.3	0.22 (8)	4.16	10- 4	-308 / 266	0.03 (1)
13- 1	-4811 / 1669	0.0	0.0	0.15 (1)	6.66	10- 5	-1213 / 3435	0.37 (3)
7- 6	-4785 / 1667	0.0	0.0	0.16 (3)	6.68	8- 5	-132 / 414	0.04 (4)
						1-12	-1529 / 4724	0.51 (1)
13-12	-136 / 150	-440.6	-440.6	0.21 (3)	6.25	8- 6	-1527 / 4732	0.51 (3)
12-11	-1454 / 4459	-440.6	-440.6	0.46 (3)	6.25			
11-10	-2311 / 7006	-440.6	-440.6	0.56 (3)	6.25			
10- 9	-1402 / 4430	-440.6	-440.6	0.45 (3)	6.25			
9- 8	-1402 / 4430	-440.6	-440.6	0.45 (3)	6.25			
8- 7	-12 / 26	-440.6	-440.6	0.20 (3)	6.25			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

RECEIVED
TOWN OF MILTON
MAR 29, 2017
17-4978
BUILDING DIVISION

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

GIRDER TYPE: CStdGirder
START DISTANCE = 5-8
START SPAN CARRIED = 19-11-10
END DISTANCE = 18-10-8
END SPAN CARRIED = 19-11-10
END WALL WIDTH = 1-8
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.25 (2-3:7) , BC=0.56 (10-11:3) , WB=0.51 (1-12:1) , SSI=0.31 (12-13:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (2) (INPUT = 0.90)
JSI METAL= 0.68 (9) (INPUT = 1.00)

CONTINUED ON PAGE 2

PLATES (table is in inches)

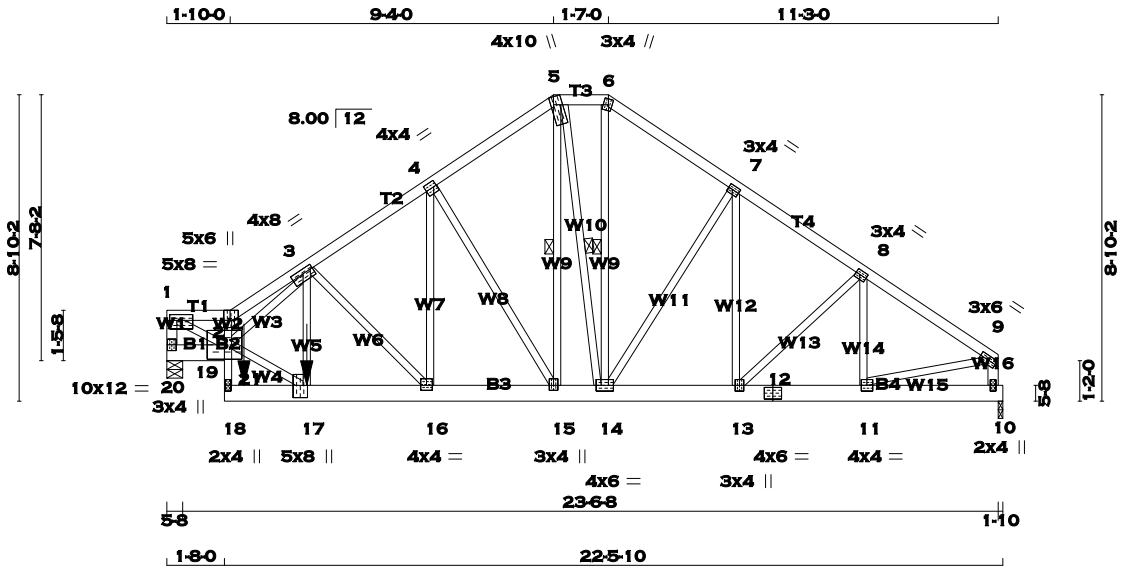
JT	TYPE	PLATES	W	LEN	Y	X
7	BMV1+p	MT20	3.0	4.0		
8	BMWW-t	MT20	5.0	5.0	2.50	1.50
9	BS-t	MT20	4.0	5.0		
10	BMWW-t	MT20	4.0	5.0	2.00	1.50
11	BMWWW-t	MT20	4.0	8.0	2.00	3.50
12	BMWW-t	MT20	5.0	5.0	2.50	1.50
13	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



TOTAL WEIGHT = 3 X 140 = 421 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
20 - 1	2x4	DRY	No.2	SPF
1 - 2	2x4	DRY	No.2	SPF
2 - 5	2x4	DRY	No.2	SPF
5 - 6	2x4	DRY	No.2	SPF
6 - 9	2x4	DRY	No.2	SPF
10 - 9	2x4	DRY	No.2	SPF
20 - 19	2x6	DRY	No.2	SPF
18 - 2	2x3	DRY	No.2	SPF
18 - 12	2x6	DRY	No.2	SPF
12 - 10	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

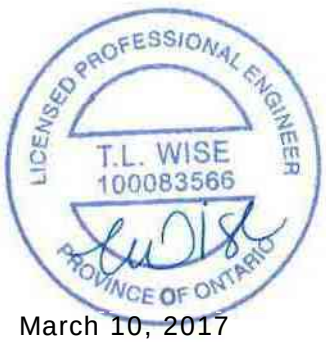
CHORDS #ROWS	SURFACE SPACING (IN)		LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
20- 1	1	12	TOP
1- 2	1	12	TOP
2- 5	1	12	TOP
5- 6	1	12	TOP
6- 9	1	12	TOP
10- 9	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
20- 19	2	12	SIDE(90.3)
18- 12	3	4	SIDE(1746.3)
12- 10	2	12	TOP
2- 18	1	12	SIDE(42.9)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.



March 10, 2017

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
20	6209	0	7097	441	-3140	5-8	5-8
10	2037	0	2218	0	-976	1-10	1-10

PROVIDE ANCHORAGE AT BEARING JOINT 20 FOR 3140 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 10 FOR 976 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 441 LBS FACTORED HORIZONTAL REACTION AT JOINT 20

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
20	4358	3048 / 0	0 / 0	0 / 0	2219 / -3084	1309 / 0	0 / 0
10	1429	1000 / 0	0 / 0	0 / 0	453 / -973	429 / 0	0 / 0
HORIZONTAL REACTIONS							
20	---	0 / 0	0 / 0	0 / 0	315 / -323	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 20, 10

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.26 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 4.05 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 5-15. DBS = 16-0-0 . CBF = 88 LBS.
1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 5-14, 6-14. DBS = 20-0-0 . CBF = 74 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) TO EACH PLY USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
20- 1	-6961 / 3113	0.0	0.0	0.21 (1)	5.67	1-19	-5941 / 13388	0.88 (1)
1- 2	-11434 / 5091	-77.3	-77.3	0.25 (7)	3.61	19-17	-3271 / 7264	0.47 (3)
2- 3	-13786 / 6209	-77.3	-77.3	0.32 (7)	3.26	19- 3	-3272 / 7174	0.47 (3)
3- 4	-4628 / 2148	-77.3	-77.3	0.15 (7)	5.29	17- 3	-1175 / 2390	0.16 (3)
4- 5	-2877 / 1461	-77.3	-77.3	0.11 (7)	6.25	3-16	-4184 / 1990	0.56 (3)
5- 6	-2305 / 1234	-77.3	-77.3	0.07 (8)	6.25	16- 4	-1402 / 3112	0.29 (7)
6- 7	-2769 / 1392	-77.3	-77.3	0.11 (8)	6.25	4-15	-2908 / 1487	0.89 (3)
7- 8	-2824 / 1347	-77.3	-77.3	0.11 (8)	6.25	15- 5	-986 / 2022	0.15 (7)
8- 9	-2697 / 1206	-77.3	-77.3	0.11 (8)	6.25	5-14	-629 / 342	0.10 (3)
10- 9	-2174 / 989	0.0	0.0	0.07 (1)	7.81	14- 6	-661 / 1353	0.10 (7)
						14- 7	-207 / 271	0.06 (4)
20-19	-421 / 200	-134.1	-134.1	0.02 (3)	6.25	13- 7	-181 / 163	0.04 (3)
18-19	-451 / 1041	0.0	0.0	0.39 (3)	7.81	13- 8	-108 / 131	0.01 (6)
19- 2	-821 / 39	0	0	0	4.05	11- 8	-470 / 272	0.03 (3)
18-21	/ 34	-1	1	0.1 (3)	6.25	11- 9	-939 / 2322	0.16 (1)
21-17	/ 34	17	-	5 0.1 (3)	6.25			
17-16	305 / 6	17	-	5 0.2 (3)	6.25			
16-15	1057 / 38	17	-	17.5 0.1 (3)	6.25			
15-14	-900 / 2419	-17.5	-17.5	0.16 (1)	6.25			
14-13								
13-12								
12-11								
11-10								

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP	CH.	LL =	23.3	PSF
		DL =	3.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.0	PSF
TOTAL	LOAD	=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 6-11-0
END DISTANCE = 2-2-12
END SPAN CARRIED = 6-11-0
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.80")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.80")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.32 (2-3:7) , BC=0.44 (2-19:3) , WB=0.89 (4-15:3) , SSI=0.18 (17-18:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (16) (INPUT = 0.90)
JSI METAL= 0.71 (1) (INPUT = 1.00)

CONTINUED ON PAGE 2

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
1	TMVW-t	MT20	5.0	8.0	2.00	2.50
2	TTV+p	MT20	5.0	6.0	Edge	2.25
3	TMWWW-t	MT20	4.0	8.0	1.50	3.75
4	TMWW-t	MT20	4.0	4.0	2.00	1.00
5	TTWW+m	MT20	4.0	10.0	Edge	1.00
6	TTW+m	MT20	3.0	4.0	2.00	1.25
7	TMWW-t	MT20	3.0	4.0	1.50	1.50
8	TMWW-t	MT20	3.0	4.0	1.50	1.50
9	TMVW-t	MT20	3.0	6.0		
10	BMV1+p	MT20	2.0	4.0		
11	BMWW-t	MT20	4.0	4.0		
12	BS-t	MT20	4.0	6.0		
13	BMWW+t	MT20	3.0	4.0		
14	BMWWW-t	MT20	4.0	6.0		
15	BMWW+t	MT20	3.0	4.0	1.75	1.50
16	BMWW-t	MT20	4.0	4.0	1.75	2.00
17	BMWW+t	MT20	5.0	8.0	4.25	1.50
18	BMV+p	MT20	2.0	4.0		
19	BVMWWW-l	MT20	10.0	12.0		
20	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1155.3 lbs FACTORED DOWN AND 524.6 lbs FACTORED UP AT 2-2-12, AND 5476.7 lbs FACTORED DOWN AND 2486.8 lbs FACTORED UP AT 4-0-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

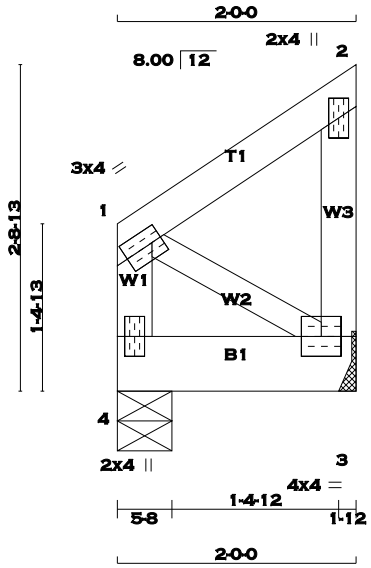
FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
17	4-0-8	-4715	-5477	2487	FRONT	VERT	TOTAL
21	2-2-12	-995	-1155	525	FRONT	VERT	TOTAL

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



SCALE = 1:19.3

TOTAL WEIGHT = 11 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
4 - 1	2x4	DRY	No.2	SPF
1 - 2	2x4	DRY	No.2	SPF
3 - 2	2x4	DRY	No.2	SPF
4 - 3	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
1	TMVW-t	MT20	3.0	4.0	1.50	1.00
2	TMV+p	MT20	2.0	4.0		
3	BMVW1-t	MT20	4.0	4.0		
4	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
4	222	0	267	138	-86	5-8	5-8
3	222	0	271	0	-151	HANGER BY OTHERS MIN. SEAT SIZE: 1-12	

PROVIDE ANCHORAGE AT BEARING JOINT 4 FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 3 FOR 151 LBS FACTORED UPLIFT

PROVIDE FOR 138 LBS FACTORED HORIZONTAL REACTION AT JOINT 4

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
4	156	109 / 0	0 / 0	0 / 0	111 / -91	47 / 0	0 / 0
3	156	109 / 0	0 / 0	0 / 0	121 / -138	47 / 0	0 / 0
HORIZONTAL REACTIONS							
4	---	0 / 0	0 / 0	0 / 0	98 / -76	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 4

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
4- 1	-100 / 52	0.0	0.0	0.01 (8)	7.81	1- 3	-57 / 108	0.02 (6)
1- 2	-59 / 53	-77.3	-77.3	0.06 (3)	6.25			
3- 2	-92 / 69	0.0	0.0	0.04 (7)	7.81			
4- 3	-118 / 97	-145.0	-145.0	0.06 (3)	6.25			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	23.3	PSF
		DL =	3.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.0	PSF
TOTAL	LOAD	=	33.3	PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 7-4-8
END DISTANCE = 2-0-0
END SPAN CARRIED = 7-4-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.06 (1-2:3) , BC=0.06 (3-4:3) , WB=0.02 (1-3:6) , SSI=0.10 (3-4:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.31 (1) (INPUT = 0.90)
JSI METAL= 0.04 (3) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
T.L. WISE
100083566
PROVINCE OF ONTARIO

March 10, 2017

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

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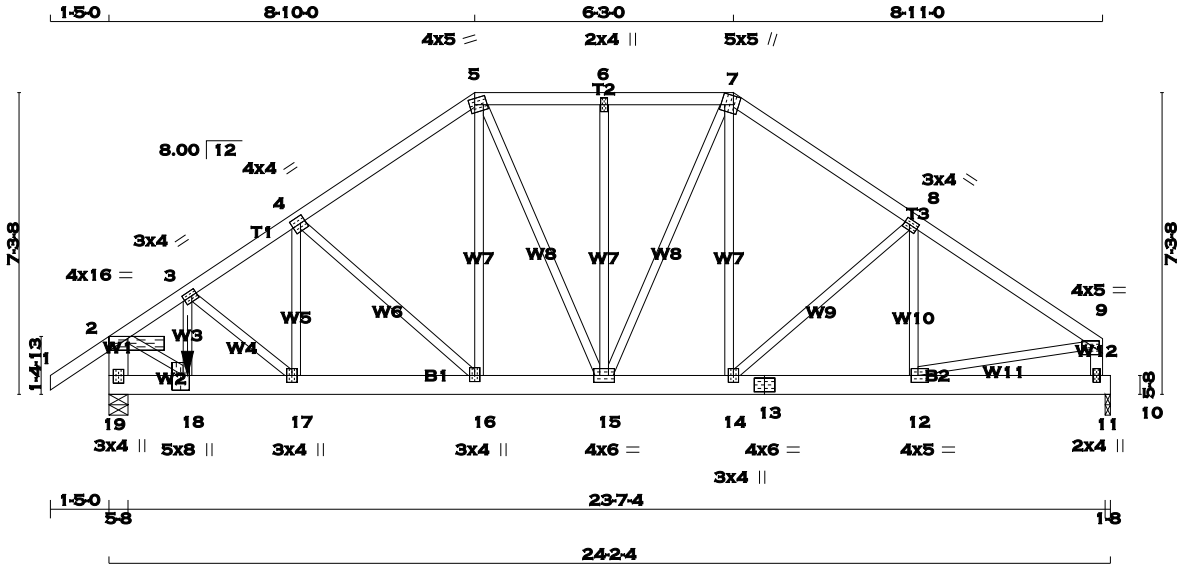

TOTAL WEIGHT = 131 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 5	2x4	DRY	No.2	SPF
5 - 7	2x4	DRY	No.2	SPF
7 - 9	2x4	DRY	No.2	SPF
19 - 2	2x6	DRY	No.2	SPF
11 - 9	2x4	DRY	No.2	SPF
19 - 13	2x6	DRY	No.2	SPF
13 - 10	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-p	MT20	4.0	16.0	Edge	5.50
3	TMWW-t	MT20	3.0	4.0	1.50	1.50
4	TMWW-t	MT20	4.0	4.0	2.00	1.50
5	TTWW-m	MT20	4.0	5.0	1.75	1.50
6	TMW+w	MT20	2.0	4.0		
7	TTWW+m	MT20	5.0	5.0	2.50	1.50
8	TMWW-t	MT20	3.0	4.0	1.50	1.50
9	TMVW-p	MT20	4.0	5.0	1.25	2.50
11	BMV+p	MT20	2.0	4.0		
12	BMWW-t	MT20	4.0	5.0	2.00	2.00
13	BS-t	MT20	4.0	6.0		
14	BMWW+t	MT20	3.0	4.0		
15	BMWWW-t	MT20	4.0	6.0		
16	BMWW+t	MT20	3.0	4.0	1.75	1.50
17	BMWW+t	MT20	3.0	4.0		
18	BMWW+t	MT20	5.0	8.0	4.25	1.75
19	BMV1+p	MT20	3.0	4.0	2.25	1.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1863.7 lbs FACTORED DOWN AND 885.3 lbs FACTORED UP AT 1-10-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
T.L. WISE
100083566
PROVINCE OF ONTARIO
March 10, 2017

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
19	2813	0	3066	314	-1422	5-8	5-8
10	1261	0	1334	0	-595	1-8	1-8

PROVIDE ANCHORAGE AT BEARING JOINT 19 FOR 1422 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 10 FOR 595 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 314 LBS FACTORED HORIZONTAL REACTION AT JOINT 19

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
19	1972	1394 / 0	0 / 0	0 / 0	631 / -1387	577 / 0	0 / 0
10	885	617 / 0	0 / 0	0 / 0	183 / -597	268 / 0	0 / 0
HORIZONTAL REACTIONS							
19	---	0 / 0	0 / 0	0 / 0	224 / -216	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 19, 10

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.05 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
1- 2	0 / 32	-77.3	-77.3	0.14 (1)	10.00
2- 3	-2729 / 1294	-77.3	-77.3	0.30 (7)	4.05
3- 4	-2197 / 1065	-77.3	-77.3	0.37 (7)	4.35
4- 5	-1585 / 838	-77.3	-77.3	0.34 (7)	4.99
5- 6	-1333 / 773	-77.3	-77.3	0.21 (8)	5.48
6- 7	-1333 / 773	-77.3	-77.3	0.21 (8)	5.48
7- 8	-1442 / 784	-77.3	-77.3	0.41 (8)	5.08
8- 9	-1635 / 765	-77.3	-77.3	0.41 (8)	4.82
19- 2	-2935 / 1377	0.0	0.0	0.19 (1)	6.16
11- 9	-1355 / 644	0.0	0.0	0.14 (1)	6.99
19-18	-294 / 293	-17.5	-17.5	0.15 (3)	6.25
18-17	-1126 / 2297	-17.5	-17.5	0.41 (1)	6.25
17-16	-887 / 1907	-17.5	-17.5	0.25 (1)	6.25
16-15	-496 / 1329	-17.5	-17.5	0.17 (1)	6.25
15-14	-379 / 1186	-17.5	-17.5	0.16 (1)	6.25
14-13	-519 / 1369	-17.5	-17.5	0.20 (1)	6.25
13-12	-519 / 1369	-17.5	-17.5	0.20 (1)	6.25
12-11	-9 / 18	-17.5	-17.5	0.17 (1)	10.00
11-10	0 / 0	-17.5	-17.5	0.17 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
17- 4	-193 / 486	0.11 (1)
4-16	-794 / 535	0.47 (3)
16- 5	-307 / 592	0.27 (7)
5-15	-116 / 90	0.13 (8)
15- 6	-296 / 231	0.26 (3)
15- 7	-252 / 369	0.27 (7)
14- 7	-160 / 272	0.14 (8)
14- 8	-331 / 315	0.20 (4)
12- 8	-132 / 151	0.03 (3)
2-18	-1060 / 2507	0.57 (1)
12- 9	-549 / 1397	0.33 (1)
18- 3	-284 / 545	0.12 (4)
3-17	-515 / 315	0.11 (3)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
18	1-10-12	-1679	-1864	885	FRONT	VERT	TOTAL

WIND LOADS APPLIED IS BASED ON REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0} FT-SX ABOVE GROUND HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, GCpg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}. INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MATERIAL TO BE AT LEAST {0-0} FT-IN-SX AWAY {OPE
FRO

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

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DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP	CH.	LL =	23.3	PSF
		DL =	3.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.0	PSF
TOTAL	LOAD	=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.41 (7-8:8) , BC=0.41 (17-18:1) , WB=0.57 (2-18:1) , SSI=0.84 (10-11:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

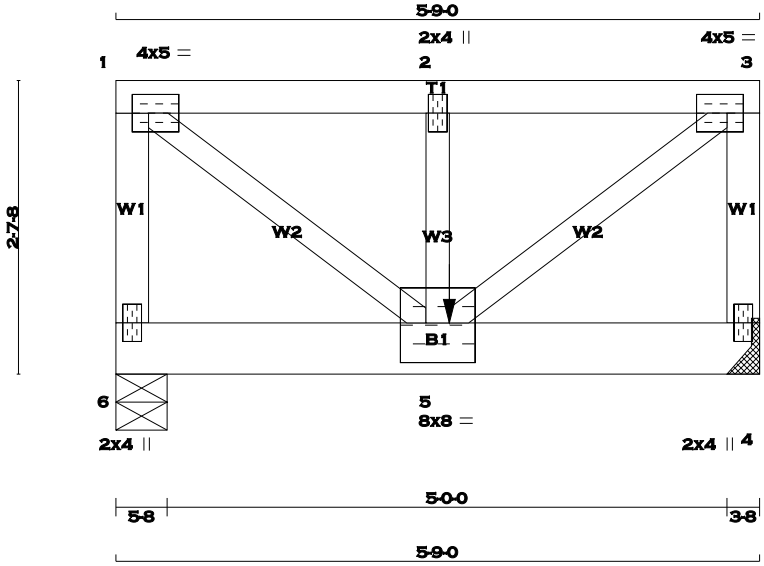
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (18) (INPUT = 0.90)
JSI METAL= 0.58 (18) (INPUT = 1.00)



SCALE = 1:20.6

TOTAL WEIGHT = 28 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
6 - 1	2x4	DRY	No.2	SPF
1 - 3	2x4	DRY	No.2	SPF
4 - 3	2x4	DRY	No.2	SPF
6 - 4	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
1	TMVW-t	MT20	4.0	5.0	2.00	1.75
2	TMW+w	MT20	2.0	4.0		
3	TMVW-t	MT20	4.0	5.0	2.00	1.75
4	BMV1+p	MT20	2.0	4.0		
5	BMWWW-t	MT20	8.0	8.0	4.25	4.00
6	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 528.0 lbs FACTORED DOWN AND 239.7 lbs FACTORED UP AT 2-11-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
6	1302	0	1473	-115	-554	5-8	5-8
4	1679	0	1924	0	-690	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

PROVIDE ANCHORAGE AT BEARING JOINT 6 FOR 554 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 4 FOR 690 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 115 LBS FACTORED HORIZONTAL REACTION AT JOINT 6

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
6	914	639 / 0	0 / 0	0 / 0	429 / -572	275 / 0	0 / 0
4	1178	824 / 0	0 / 0	0 / 0	613 / -721	354 / 0	0 / 0
HORIZONTAL REACTIONS							
6	---	0 / 0	0 / 0	0 / 0	82 / -82	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 6

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.45 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
6- 1	-1165 / 472	0.0	0.0	0.14 (1)	7.38	1- 5	-645 / 1720	0.38 (1)
1- 2	-1372 / 552	-77.3	-77.3	0.14 (7)	5.45	5- 2	-256 / 211	0.05 (7)
2- 3	-1372 / 552	-77.3	-77.3	0.14 (7)	5.45	5- 3	-645 / 1741	0.38 (1)
4- 3	-1178 / 472	0.0	0.0	0.14 (1)	7.36			
6- 5	-74 / 99	-230.9	-230.9	0.14 (4)	6.25			
5- 4	-17 / 44	-493.0	-493.0	0.31 (3)	6.25			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
5	2-11-12	-455	-528	240	FRONT	VERT	TOTAL

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP	CH.	LL =	23.3	PSF
		DL =	3.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.0	PSF
TOTAL	LOAD	=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 11-5-8
END DISTANCE = 2-11-12
END SPAN CARRIED = 11-5-8
END WALL WIDTH = 5-8
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

GIRDER TYPE: CStdGirder
START DISTANCE = 2-11-12
START SPAN CARRIED = 22-2-2
END DISTANCE = 5-9-0
END SPAN CARRIED = 22-2-2
END WALL WIDTH = 1-8
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.14 (2-3:7) , BC=0.31 (4-5:3) , WB=0.38 (1-5:1) , SSI=0.52 (4-5:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

LICENSED PROFESSIONAL ENGINEER
T.L. WISE
100083566
PROVINCE OF ONTARIO

March 10, 2017

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

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CONTINUED ON PAGE 2

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

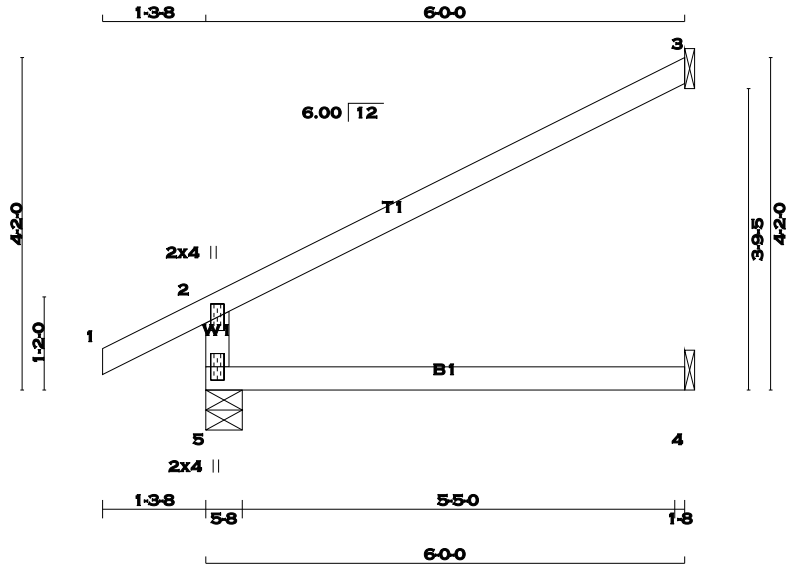
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (1) (INPUT = 0.90)
JSI METAL= 0.41 (3) (INPUT = 1.00)

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

SCALE = 1:28.9



TOTAL WEIGHT = 22 X 17 = 376 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
5 - 2	2x4	DRY	No.2	SPF
1 - 3	2x4	DRY	No.2	SPF
5 - 4	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMV+p	MT20	2.0	4.0		
5	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
5	457	0	465	273	-210	5-8	5-8
3	174	0	185	0	-179	1-8	1-8
4	43	0	49	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) 3 , 4

PROVIDE ANCHORAGE AT BEARING JOINT 5 FOR 210 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 3 FOR 179 LBS FACTORED UPLIFT

PROVIDE FOR 273 LBS FACTORED HORIZONTAL REACTION AT JOINT 5

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
5	318	238 / 0	0 / 0	0 / 0	22 / -202	80 / 0	0 / 0
3	118	105 / 0	0 / 0	0 / 0	28 / -137	14 / 0	0 / 0
4	35	0 / 0	0 / 0	0 / 0	1 / -4	35 / 0	0 / 0
HORIZONTAL REACTIONS							
5	---	0 / 0	0 / 0	0 / 0	195 / -62	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 5

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
5- 2	-404 / 260	0.0	0.0	0.21 (7)	7.81
1- 2	0 / 23	-77.3	-77.3	0.10 (1)	10.00
2- 3	-87 / 3	-77.3	-77.3	0.47 (1)	6.25
5- 4	0 / 0	-17.5	-17.5	0.13 (11)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.47 (2-3:1) , BC=0.13 (4-5:11) , WB=0.00 (n/a:0) , SSI=0.20 (2-3:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

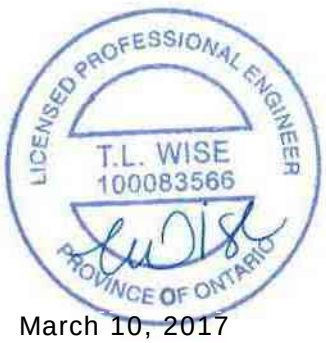
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

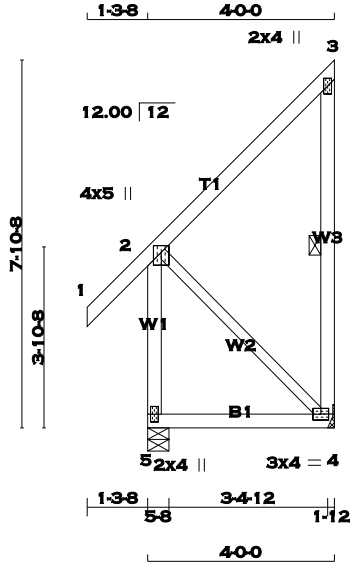
JSI GRIP= 0.33 (2) (INPUT = 0.90)
JSI METAL= 0.18 (5) (INPUT = 1.00)



March 10, 2017

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



SCALE = 1:49.3

TOTAL WEIGHT = 4 X 30 = 118 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 3	2x4	DRY	No.2	SPF
4 - 3	2x4	DRY	No.2	SPF
5 - 2	2x4	DRY	No.2	SPF
5 - 4	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW+p	MT20	4.0	5.0	1.75	2.00
3	TMV+p	MT20	2.0	4.0		
4	BMVW1-t	MT20	3.0	4.0		
5	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
4	190	0	338	0	-501	IN-SX	IN-SX
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-12	
5	298	0	415	480	-251	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 4 FOR 501 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 5 FOR 251 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 480 LBS FACTORED HORIZONTAL REACTION AT JOINT 5

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
4	133	93 / 0	0 / 0	0 / 0	272 / -384	40 / 0	0 / 0
5	206	158 / 0	0 / 0	0 / 0	294 / -210	48 / 0	0 / 0
HORIZONTAL REACTIONS							
5	---	0 / 0	0 / 0	0 / 0	343 / -262	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 5

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 3-4. DBS = 20-0-0 . CBF = 21 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 38	-77.3	-77.3	0.11 (7)	10.00	2- 4	-250 / 456	0.11 (5)
2- 3	-216 / 201	-77.3	-77.3	0.31 (7)	6.25			
4- 3	-197 / 226	0.0	0.0	0.34 (7)	6.25			
5- 2	-380 / 276	0.0	0.0	0.09 (4)	7.81			
5- 4	-411 / 335	-17.5	-17.5	0.08 (11)	6.25			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.34 (3-4:7) , BC=0.08 (4-5:11) , WB=0.11 (2-4:5) , SSI=0.13 (2-3:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.63 (4) (INPUT = 0.90)
JSI METAL= 0.15 (4) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
T.L. WISE
100083566
PROVINCE OF ONTARIO

March 10, 2017

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

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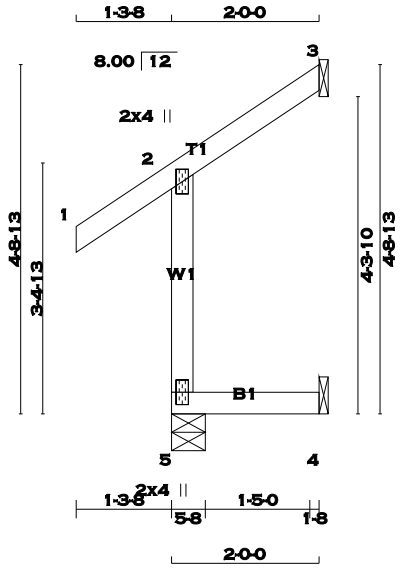
MAR 29, 2017
17-4978
BUILDING DIVISION



SCALE = 1:31.2

TOTAL WEIGHT = 4 X 10 = 42 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
5 - 2	2x4	DRY	No.2	SPF
1 - 3	2x4	DRY	No.2	SPF
5 - 4	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMV+p	MT20	2.0	4.0		
5	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
5	220	0	220	259	0	5-8	5-8
3	59	0	71	0	-70	1-8	1-8
4	17	0	49	0	-127	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) 3 , 4

PROVIDE ANCHORAGE AT BEARING JOINT 3 FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 4 FOR 150 LBS FACTORED UPLIFT

PROVIDE FOR 259 LBS FACTORED HORIZONTAL REACTION AT JOINT 5

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
5	152	121 / 0	0 / 0	0 / 0	20 / -44	30 / 0	0 / 0
3	40	36 / 0	0 / 0	0 / 0	29 / -53	5 / 0	0 / 0
4	13	0 / 0	0 / 0	0 / 0	81 / -99	13 / 0	0 / 0
HORIZONTAL REACTIONS							
5	---	0 / 0	0 / 0	0 / 0	185 / -170	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 5

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (12)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
5- 2	-216 / 119	0.0	0.0	0.41 (7)	7.81			
1- 2	0 / 29	-77.3	-77.3	0.10 (1)	10.00			
2- 3	-42 / 5	-77.3	-77.3	0.05 (7)	6.25			
5- 4	0 / 0	-17.5	-17.5	0.33 (7)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.41 (2-5:7) , BC=0.33 (4-5:7) , WB=0.00 (n/a:0) , SSI=0.18 (2-5:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.26 (5) (INPUT = 0.90)
JSI METAL= 0.17 (5) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
T.L. WISE
100083566
PROVINCE OF ONTARIO

March 10, 2017

KOTT

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TOWN OF MILTON
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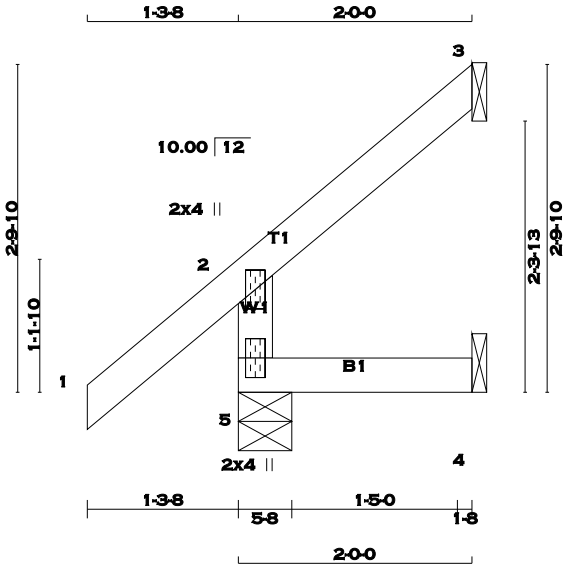
17-4978
BUILDING DIVISION



SCALE = 1:19.7

TOTAL WEIGHT = 6 X 8 = 50 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
5 - 2	2x4	DRY	No.2	SPF
1 - 3	2x4	DRY	No.2	SPF
5 - 4	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
2	TMV+p	MT20	2.0	4.0	
5	BMV1+p	MT20	2.0	4.0	

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
5	223	0	232	205	-73	5-8	5-8
3	58	0	72	0	-82	1-8	1-8
4	16	0	19	0	-10	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) 3 , 4

PROVIDE ANCHORAGE AT BEARING JOINT 5 FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 3 FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 4 FOR 150 LBS FACTORED UPLIFT

PROVIDE FOR 205 LBS FACTORED HORIZONTAL REACTION AT JOINT 5

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
5	154	122 / 0	0 / 0	0 / 0	24 / -72	31 / 0	0 / 0
3	40	35 / 0	0 / 0	0 / 0	33 / -62	5 / 0	0 / 0
4	12	0 / 0	0 / 0	0 / 0	8 / -15	12 / 0	0 / 0
HORIZONTAL REACTIONS							
5	---	0 / 0	0 / 0	0 / 0	147 / -87	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 5

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (12)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
5- 2	-216 / 108	0.0	0.0	0.14 (7)	7.81			
1- 2	0 / 34	-77.3	-77.3	0.11 (1)	10.00			
2- 3	-56 / 7	-77.3	-77.3	0.07 (7)	6.25			
5- 4	0 / 0	-17.5	-17.5	0.05 (7)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.14 (2-5:7) , BC=0.05 (4-5:7) , WB=0.00 (n/a:0) , SSI=0.14 (2-5:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (2) (INPUT = 0.90)
JSI METAL= 0.13 (5) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
T.L. WISE
100083566
PROVINCE OF ONTARIO
March 10, 2017

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

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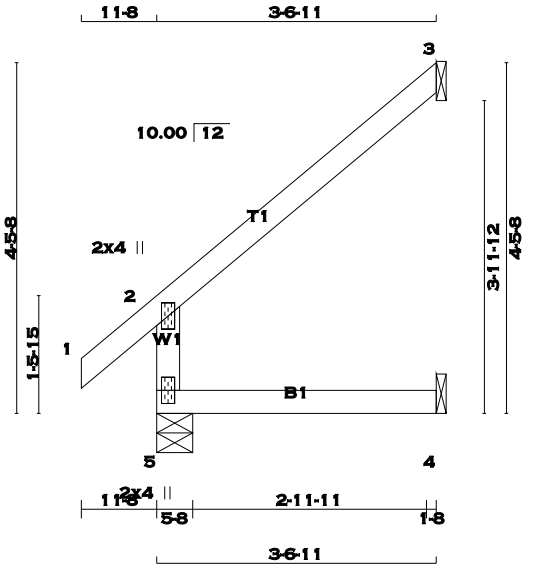

SCALE = 1:29.3

TOTAL WEIGHT = 8 X 12 = 98 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
5 - 2	2x4	DRY	No.2	SPF
1 - 3	2x4	DRY	No.2	SPF
5 - 4	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
2	TMV+p	MT20	2.0	4.0	
5	BMV1+p	MT20	2.0	4.0	

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
5	288	0	302	293	-78	5-8	5-8
3	103	0	127	0	-147	1-8	1-8
4	27	0	31	0	-3	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) 3 , 4

PROVIDE ANCHORAGE AT BEARING JOINT 5 FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 3 FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 4 FOR 150 LBS FACTORED UPLIFT

PROVIDE FOR 293 LBS FACTORED HORIZONTAL REACTION AT JOINT 5

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
5	200	152 / 0	0 / 0	0 / 0	34 / -86	48 / 0	0 / 0
3	70	62 / 0	0 / 0	0 / 0	59 / -110	8 / 0	0 / 0
4	22	0 / 0	0 / 0	0 / 0	9 / -16	22 / 0	0 / 0
HORIZONTAL REACTIONS							
5	---	0 / 0	0 / 0	0 / 0	209 / -133	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 5

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
5- 2	-271 / 126	0.0	0.0	0.28 (7)	7.81			
1- 2	0 / 26	-77.3	-77.3	0.06 (1)	10.00			
2- 3	-100 / 13	-77.3	-77.3	0.21 (7)	6.25			
5- 4	0 / 0	-17.5	-17.5	0.11 (7)	10.00			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.28 (2-5:7) , BC=0.11 (4-5:7) , WB=0.00 (n/a:0) , SSI=0.20 (2-5:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.30 (2) (INPUT = 0.90)
JSI METAL= 0.19 (5) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
T.L. WISE
100083566
PROVINCE OF ONTARIO

March 10, 2017

KOTT

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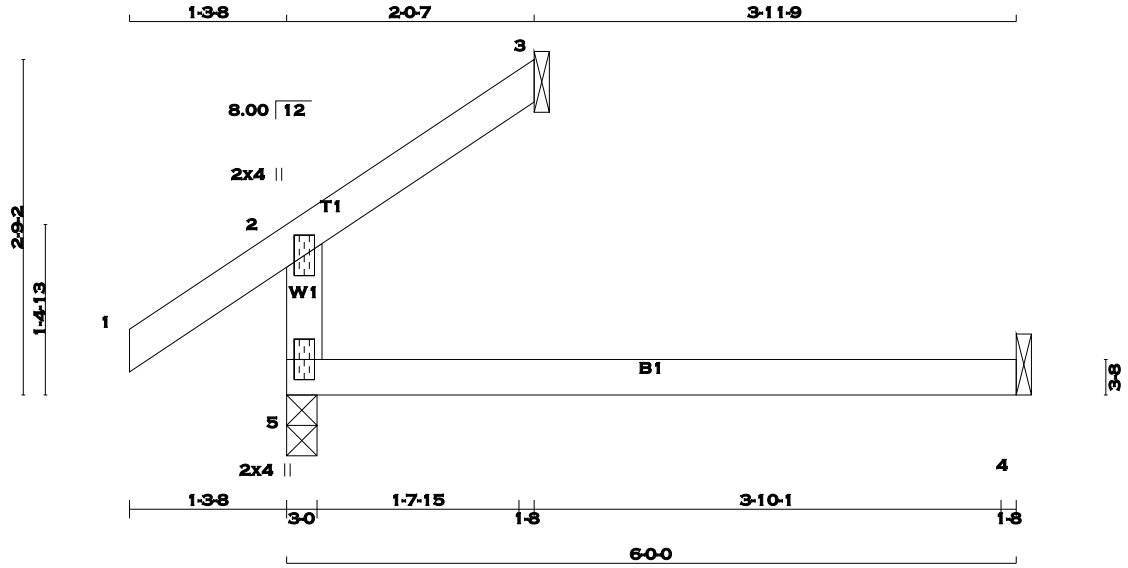

TOTAL WEIGHT = 2 X 13 = 25 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
5 - 2	2x4	DRY	No.2	SPF
1 - 3	2x4	DRY	No.2	SPF
5 - 4	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMV+p	MT20	2.0	4.0		
5	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
5	265	0	279	183	-72	3-0	3-0
3	60	0	71	0	-71	1-8	1-8
4	44	0	49	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) 3 , 4

PROVIDE ANCHORAGE AT BEARING JOINT 5 FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 3 FOR 150 LBS FACTORED UPLIFT

PROVIDE FOR 183 LBS FACTORED HORIZONTAL REACTION AT JOINT 5

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
5	188	123 / 0	0 / 0	0 / 0	35 / -93	65 / 0	0 / 0
3	40	36 / 0	0 / 0	0 / 0	29 / -54	5 / 0	0 / 0
4	35	0 / 0	0 / 0	0 / 0	2 / -4	35 / 0	0 / 0
HORIZONTAL REACTIONS							
5	---	0 / 0	0 / 0	0 / 0	130 / -88	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 5, 3

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
5- 2	-219 / 121	0.0	0.0	0.14 (7)	7.81			
1- 2	0 / 29	-77.3	-77.3	0.10 (1)	10.00			
2- 3	-42 / 5	-77.3	-77.3	0.04 (7)	6.25			
5- 4	0 / 0	-17.5	-17.5	0.13 (11)	10.00			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.14 (2-5:7) , BC=0.13 (4-5:11) , WB=0.00 (n/a:0) , SSI=0.12 (2-5:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (5) (INPUT = 0.90)
JSI METAL= 0.12 (5) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
T.L. WISE
100083566
PROVINCE OF ONTARIO

March 10, 2017

KOTT

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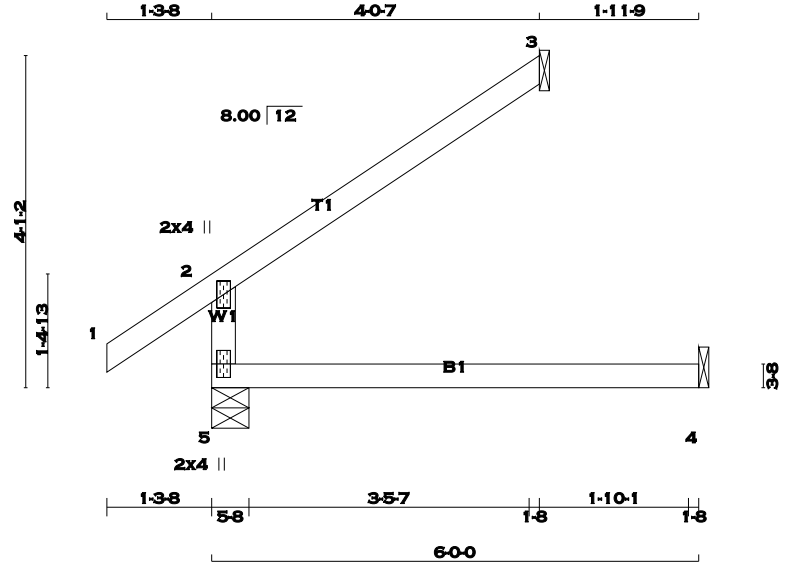


TOTAL WEIGHT = 2 X 15 = 31 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
5 - 2	2x4	DRY	No.2	SPF
1 - 3	2x4	DRY	No.2	SPF
5 - 4	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMV+p	MT20	2.0	4.0		
5	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
5	362	0	385	273	-125	5-8	5-8
3	117	0	140	0	-141	1-8	1-8
4	44	0	49	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) 3 , 4

PROVIDE ANCHORAGE AT BEARING JOINT 5 FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 3 FOR 150 LBS FACTORED UPLIFT

PROVIDE FOR 273 LBS FACTORED HORIZONTAL REACTION AT JOINT 5

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
5	254	181 / 0	0 / 0	0 / 0	58 / -135	72 / 0	0 / 0
3	80	71 / 0	0 / 0	0 / 0	57 / -106	9 / 0	0 / 0
4	35	0 / 0	0 / 0	0 / 0	3 / -6	35 / 0	0 / 0
HORIZONTAL REACTIONS							
5	---	0 / 0	0 / 0	0 / 0	195 / -123	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 5, 3

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
5- 2	-326 / 177	0.0	0.0	0.25 (7)	7.81			
1- 2	0 / 29	-77.3	-77.3	0.10 (1)	10.00			
2- 3	-84 / 11	-77.3	-77.3	0.22 (7)	6.25			
5- 4	0 / 0	-17.5	-17.5	0.13 (11)	10.00			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.25 (2-5:7) , BC=0.13 (4-5:11) , WB=0.00 (n/a:0) , SSI=0.19 (2-5:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

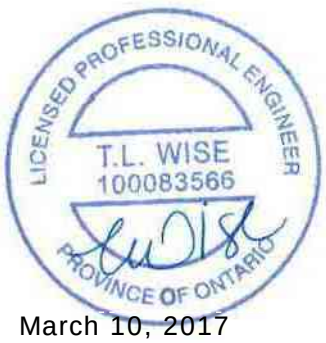
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.29 (2) (INPUT = 0.90)
JSI METAL= 0.18 (5) (INPUT = 1.00)



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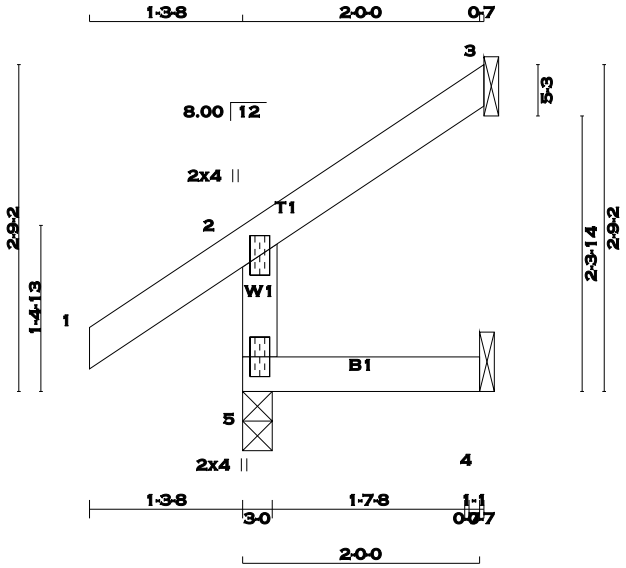

SCALE = 1:19.4

TOTAL WEIGHT = 3 X 8 = 24 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
5 - 2	2x4	DRY	No.2	SPF
1 - 3	2x4	DRY	No.2	SPF
5 - 4	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
2	TMV+p	MT20	2.0	4.0	
5	BMV1+p	MT20	2.0	4.0	

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
5	223	0	233	183	-79	3-0	3-0
3	60	0	71	0	-71	1-8	1-8
4	16	0	21	0	-17	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) 3 , 4

PROVIDE ANCHORAGE AT BEARING JOINT 5 FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 3 FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 4 FOR 150 LBS FACTORED UPLIFT

PROVIDE FOR 183 LBS FACTORED HORIZONTAL REACTION AT JOINT 5

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
5	154	123 / 0	0 / 0	0 / 0	25 / -76	31 / 0	0 / 0
3	40	36 / 0	0 / 0	0 / 0	29 / -54	5 / 0	0 / 0
4	13	0 / 0	0 / 0	0 / 0	12 / -20	13 / 0	0 / 0
HORIZONTAL REACTIONS							
5	---	0 / 0	0 / 0	0 / 0	130 / -88	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 5

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (12)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
5- 2	-219 / 121	0.0	0.0	0.15 (7)	7.81			
1- 2	0 / 29	-77.3	-77.3	0.10 (1)	10.00			
2- 3	-42 / 5	-77.3	-77.3	0.06 (7)	6.25			
5- 4	0 / 0	-17.5	-17.5	0.07 (7)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.15 (2-5:7) , BC=0.07 (4-5:7) , WB=0.00 (n/a:0) , SSI=0.12 (2-5:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (5) (INPUT = 0.90)
JSI METAL= 0.12 (5) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
T.L. WISE
100083566
PROVINCE OF ONTARIO

March 10, 2017

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

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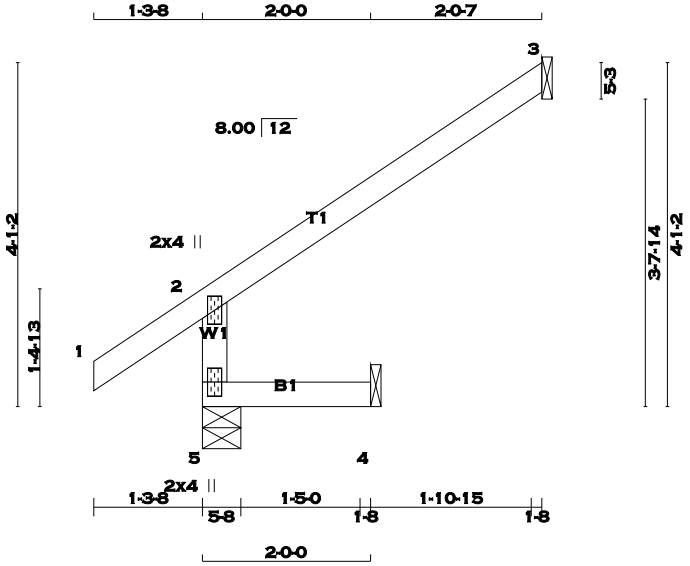

SCALE = 1:27.4

TOTAL WEIGHT = 2 X 11 = 22 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
5 - 2	2x4	DRY	No.2	SPF
1 - 3	2x4	DRY	No.2	SPF
5 - 4	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMV+p	MT20	2.0	4.0		
5	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
5	320	0	338	273	-120	5-8	5-8
3	117	0	140	0	-141	1-8	1-8
4	16	0	23	0	-32	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) 3 , 4

PROVIDE ANCHORAGE AT BEARING JOINT 5 FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 3 FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 4 FOR 150 LBS FACTORED UPLIFT

PROVIDE FOR 273 LBS FACTORED HORIZONTAL REACTION AT JOINT 5

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
5	220	181 / 0	0 / 0	0 / 0	43 / -110	39 / 0	0 / 0
3	80	71 / 0	0 / 0	0 / 0	57 / -106	9 / 0	0 / 0
4	13	0 / 0	0 / 0	0 / 0	18 / -31	13 / 0	0 / 0
HORIZONTAL REACTIONS							
5	---	0 / 0	0 / 0	0 / 0	195 / -123	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 5

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
5- 2	-326 / 177	0.0	0.0	0.24 (7)	7.81			
1- 2	0 / 29	-77.3	-77.3	0.10 (1)	10.00			
2- 3	-84 / 11	-77.3	-77.3	0.22 (7)	6.25			
5- 4	0 / 0	-17.5	-17.5	0.11 (7)	10.00			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.24 (2-5:7) , BC=0.11 (4-5:7) , WB=0.00 (n/a:0) , SSI=0.19 (2-5:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.29 (2) (INPUT = 0.90)
JSI METAL= 0.18 (5) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
T.L. WISE
100083566
PROVINCE OF ONTARIO

March 10, 2017

KOTT

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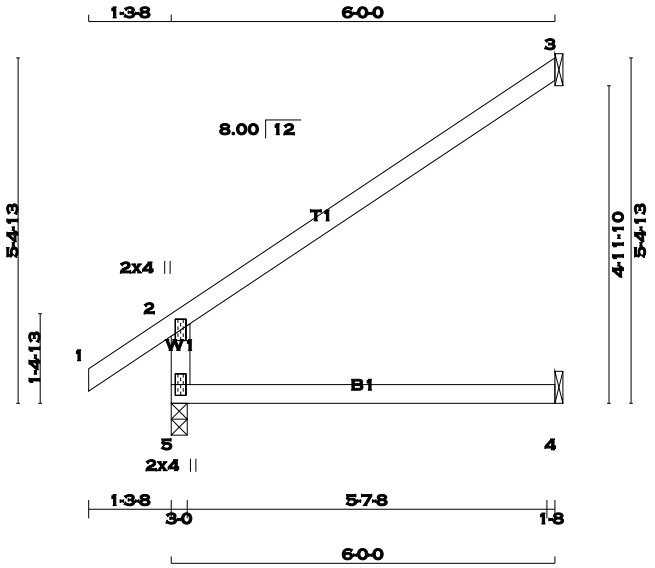

SCALE = 1:36.0

TOTAL WEIGHT = 3 X 18 = 54 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
5 - 2	2x4	DRY	No.2	SPF
1 - 3	2x4	DRY	No.2	SPF
5 - 4	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMV+p	MT20	2.0	4.0		
5	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
5	457	0	489	362	-176	3-0	3-0
3	174	0	208	0	-209	1-8	1-8
4	44	0	49	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) 3 , 4

PROVIDE ANCHORAGE AT BEARING JOINT 5 FOR 176 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 3 FOR 209 LBS FACTORED UPLIFT

PROVIDE FOR 362 LBS FACTORED HORIZONTAL REACTION AT JOINT 5

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
5	318	239 / 0	0 / 0	0 / 0	80 / -177	80 / 0	0 / 0
3	118	105 / 0	0 / 0	0 / 0	85 / -158	14 / 0	0 / 0
4	35	0 / 0	0 / 0	0 / 0	5 / -8	35 / 0	0 / 0
HORIZONTAL REACTIONS							
5	---	0 / 0	0 / 0	0 / 0	259 / -158	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 5

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
5- 2	-430 / 231	0.0	0.0	0.35 (7)	7.81			
1- 2	0 / 29	-77.3	-77.3	0.10 (1)	10.00			
2- 3	-125 / 16	-77.3	-77.3	0.38 (7)	6.25			
5- 4	0 / 0	-17.5	-17.5	0.13 (11)	10.00			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.38 (2-3:7) , BC=0.13 (4-5:11) , WB=0.00 (n/a:0) , SSI=0.25 (2-5:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.40 (2) (INPUT = 0.90)
JSI METAL= 0.24 (5) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
T.L. WISE
100083566
PROVINCE OF ONTARIO
March 10, 2017

KOTT

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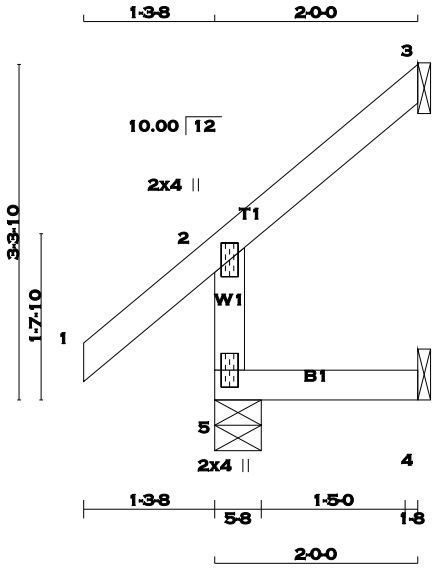
SCALE = 1:22.7



TOTAL WEIGHT = 8 X 9 = 71 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
5 - 2	2x4	DRY	No.2	SPF
1 - 3	2x4	DRY	No.2	SPF
5 - 4	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMV+p	MT20	2.0	4.0		
5	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
5	222	0	227	225	-49	5-8	5-8
3	59	0	72	0	-82	1-8	1-8
4	16	0	24	0	-33	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) 3 , 4

PROVIDE ANCHORAGE AT BEARING JOINT 5 FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 3 FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 4 FOR 150 LBS FACTORED UPLIFT

PROVIDE FOR 225 LBS FACTORED HORIZONTAL REACTION AT JOINT 5

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
5	153	122 / 0	0 / 0	0 / 0	13 / -55	31 / 0	0 / 0
3	40	35 / 0	0 / 0	0 / 0	33 / -62	5 / 0	0 / 0
4	13	0 / 0	0 / 0	0 / 0	19 / -32	13 / 0	0 / 0
HORIZONTAL REACTIONS							
5	---	0 / 0	0 / 0	0 / 0	161 / -108	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 5, 3

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (12)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
5- 2	-216 / 108	0.0	0.0	0.21 (7)	7.81			
1- 2	0 / 34	-77.3	-77.3	0.11 (1)	10.00			
2- 3	-56 / 7	-77.3	-77.3	0.06 (7)	6.25			
5- 4	0 / 0	-17.5	-17.5	0.11 (7)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.21 (2-5:7) , BC=0.11 (4-5:7) , WB=0.00 (n/a:0) , SSI=0.15 (2-5:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (5) (INPUT = 0.90)
JSI METAL= 0.15 (5) (INPUT = 1.00)

March 10, 2017

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

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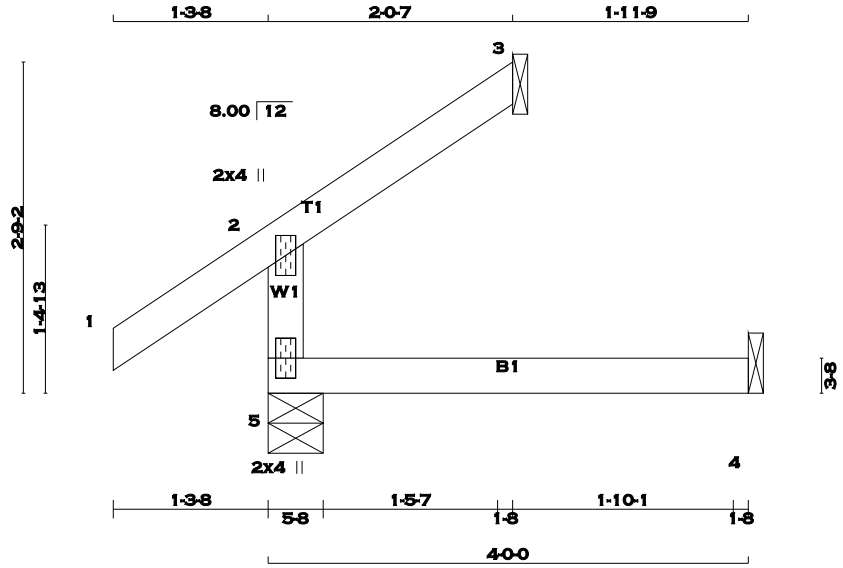

TOTAL WEIGHT = 10 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
5 - 2	2x4	DRY	No.2	SPF
1 - 3	2x4	DRY	No.2	SPF
5 - 4	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMV+p	MT20	2.0	4.0		
5	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
5	244	0	257	183	-82	5-8	5-8
3	60	0	71	0	-71	1-8	1-8
4	30	0	34	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) 3 , 4

PROVIDE ANCHORAGE AT BEARING JOINT 5 FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 3 FOR 150 LBS FACTORED UPLIFT

PROVIDE FOR 183 LBS FACTORED HORIZONTAL REACTION AT JOINT 5

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
5	171	123 / 0	0 / 0	0 / 0	33 / -89	48 / 0	0 / 0
3	40	36 / 0	0 / 0	0 / 0	29 / -54	5 / 0	0 / 0
4	24	0 / 0	0 / 0	0 / 0	4 / -7	24 / 0	0 / 0
HORIZONTAL REACTIONS							
5	---	0 / 0	0 / 0	0 / 0	130 / -88	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 5, 3

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
5- 2	-219 / 121	0.0	0.0	0.16 (7)	7.81			
1- 2	0 / 29	-77.3	-77.3	0.10 (1)	10.00			
2- 3	-42 / 5	-77.3	-77.3	0.06 (7)	6.25			
5- 4	0 / 0	-17.5	-17.5	0.06 (7)	10.00			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.16 (2-5:7) , BC=0.06 (4-5:7) , WB=0.00 (n/a:0) , SSI=0.12 (2-5:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (5) (INPUT = 0.90)
JSI METAL= 0.12 (5) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
T.L. WISE
100083566
PROVINCE OF ONTARIO

March 10, 2017

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

RECEIVED
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MAR 29, 2017
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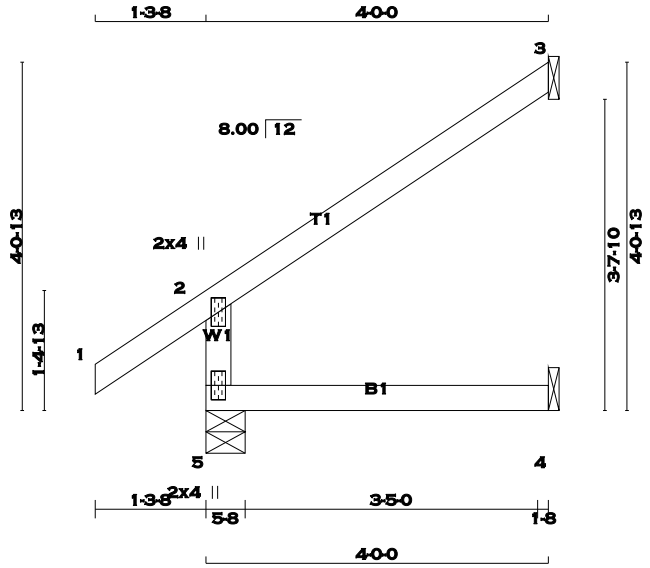

SCALE = 1:26.9

TOTAL WEIGHT = 2 X 13 = 26 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
5 - 2	2x4	DRY	No.2	SPF
1 - 3	2x4	DRY	No.2	SPF
5 - 4	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMV+p	MT20	2.0	4.0		
5	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
5	339	0	361	272	-131	5-8	5-8
3	116	0	139	0	-139	1-8	1-8
4	30	0	34	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) 3 , 4

PROVIDE ANCHORAGE AT BEARING JOINT 5 FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 3 FOR 150 LBS FACTORED UPLIFT

PROVIDE FOR 272 LBS FACTORED HORIZONTAL REACTION AT JOINT 5

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
5	235	180 / 0	0 / 0	0 / 0	54 / -129	55 / 0	0 / 0
3	79	70 / 0	0 / 0	0 / 0	57 / -105	9 / 0	0 / 0
4	24	0 / 0	0 / 0	0 / 0	7 / -11	24 / 0	0 / 0
HORIZONTAL REACTIONS							
5	---	0 / 0	0 / 0	0 / 0	194 / -123	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 5

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
5- 2	-324 / 176	0.0	0.0	0.25 (7)	7.81			
1- 2	0 / 29	-77.3	-77.3	0.10 (1)	10.00			
2- 3	-83 / 11	-77.3	-77.3	0.22 (7)	6.25			
5- 4	0 / 0	-17.5	-17.5	0.09 (7)	10.00			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.25 (2-5:7) , BC=0.09 (4-5:7) , WB=0.00 (n/a:0) , SSI=0.18 (2-5:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.29 (2) (INPUT = 0.90)
JSI METAL= 0.18 (5) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
T.L. WISE
100083566
PROVINCE OF ONTARIO

March 10, 2017

KOTT

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RECEIVED
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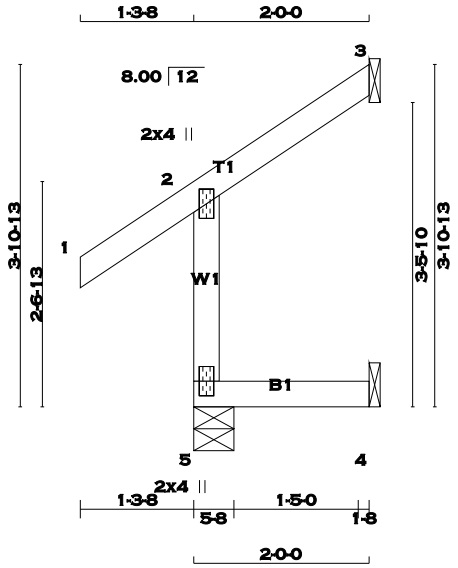

SCALE = 1:26.3

TOTAL WEIGHT = 9 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
5 - 2	2x4	DRY	No.2	SPF
1 - 3	2x4	DRY	No.2	SPF
5 - 4	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMV+p	MT20	2.0	4.0		
5	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
5	220	0	220	227	-21	5-8	5-8
3	59	0	70	0	-70	1-8	1-8
4	16	0	35	0	-74	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) 3 , 4

PROVIDE ANCHORAGE AT BEARING JOINT 5 FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 3 FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 4 FOR 150 LBS FACTORED UPLIFT

PROVIDE FOR 227 LBS FACTORED HORIZONTAL REACTION AT JOINT 5

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
5	152	121 / 0	0 / 0	0 / 0	0 / -34	30 / 0	0 / 0
3	40	36 / 0	0 / 0	0 / 0	28 / -53	5 / 0	0 / 0
4	13	0 / 0	0 / 0	0 / 0	45 / -61	13 / 0	0 / 0
HORIZONTAL REACTIONS							
5	---	0 / 0	0 / 0	0 / 0	162 / -136	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 5

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (12)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
5- 2	-216 / 120	0.0	0.0	0.30 (7)	7.81			
1- 2	0 / 29	-77.3	-77.3	0.10 (1)	10.00			
2- 3	-42 / 5	-77.3	-77.3	0.05 (7)	6.25			
5- 4	0 / 0	-17.5	-17.5	0.20 (7)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	23.3	PSF
		DL =	3.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.0	PSF
TOTAL	LOAD	=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.30 (2-5:7) , BC=0.20 (4-5:7) , WB=0.00 (n/a:0) , SSI=0.15 (2-5:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (5) (INPUT = 0.90)
JSI METAL= 0.15 (5) (INPUT = 1.00)

March 10, 2017

KOTT

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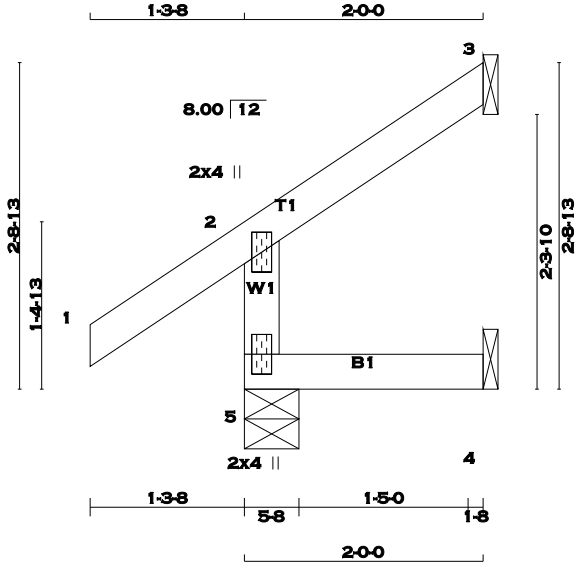

SCALE = 1:19.3

TOTAL WEIGHT = 4 X 8 = 32 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
5 - 2	2x4	DRY	No.2	SPF
1 - 3	2x4	DRY	No.2	SPF
5 - 4	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMV+p	MT20	2.0	4.0		
5	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
5	222	0	231	181	-78	5-8	5-8
3	59	0	70	0	-70	1-8	1-8
4	16	0	21	0	-16	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) 3 , 4

PROVIDE ANCHORAGE AT BEARING JOINT 5 FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 3 FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 4 FOR 150 LBS FACTORED UPLIFT

PROVIDE FOR 181 LBS FACTORED HORIZONTAL REACTION AT JOINT 5

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
5	153	122 / 0	0 / 0	0 / 0	25 / -76	31 / 0	0 / 0
3	40	35 / 0	0 / 0	0 / 0	28 / -53	5 / 0	0 / 0
4	13	0 / 0	0 / 0	0 / 0	12 / -20	13 / 0	0 / 0
HORIZONTAL REACTIONS							
5	---	0 / 0	0 / 0	0 / 0	129 / -88	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 5

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (12)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
5- 2	-217 / 120	0.0	0.0	0.15 (7)	7.81			
1- 2	0 / 29	-77.3	-77.3	0.10 (1)	10.00			
2- 3	-42 / 5	-77.3	-77.3	0.05 (7)	6.25			
5- 4	0 / 0	-17.5	-17.5	0.07 (7)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.15 (2-5:7) , BC=0.07 (4-5:7) , WB=0.00 (n/a:0) , SSI=0.12 (2-5:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (5) (INPUT = 0.90)
JSI METAL= 0.12 (5) (INPUT = 1.00)

March 10, 2017

KOTT

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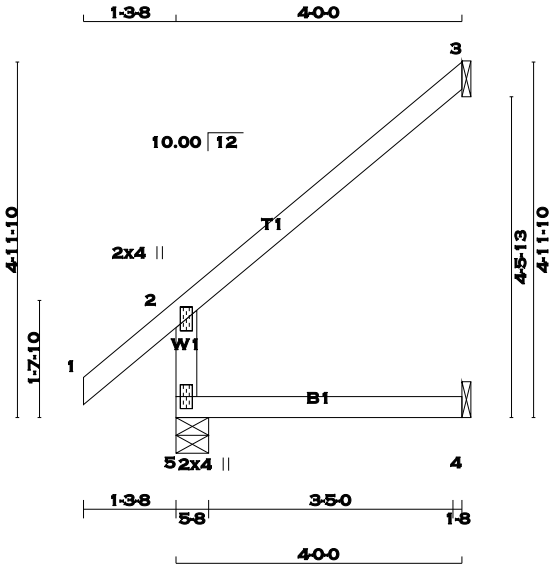

SCALE: 3/8"=1'

TOTAL WEIGHT = 5 X 14 = 70 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
5 - 2	2x4	DRY	No.2	SPF
1 - 3	2x4	DRY	No.2	SPF
5 - 4	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMV+p	MT20	2.0	4.0		
5	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
5	340	0	356	338	-97	5-8	5-8
3	116	0	143	0	-165	1-8	1-8
4	31	0	35	0	-4	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) 3 , 4

PROVIDE ANCHORAGE AT BEARING JOINT 5 FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 3 FOR 165 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 4 FOR 150 LBS FACTORED UPLIFT

PROVIDE FOR 338 LBS FACTORED HORIZONTAL REACTION AT JOINT 5

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
5	236	181 / 0	0 / 0	0 / 0	40 / -104	55 / 0	0 / 0
3	79	70 / 0	0 / 0	0 / 0	66 / -123	9 / 0	0 / 0
4	24	0 / 0	0 / 0	0 / 0	11 / -18	24 / 0	0 / 0
HORIZONTAL REACTIONS							
5	---	0 / 0	0 / 0	0 / 0	242 / -152	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 5

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
5- 2	-321 / 151	0.0	0.0	0.37 (7)	7.81			
1- 2	0 / 34	-77.3	-77.3	0.11 (1)	10.00			
2- 3	-112 / 15	-77.3	-77.3	0.26 (7)	6.25			
5- 4	0 / 0	-17.5	-17.5	0.14 (7)	10.00			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.37 (2-5:7) , BC=0.14 (4-5:7) , WB=0.00 (n/a:0) , SSI=0.23 (2-5:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.35 (2) (INPUT = 0.90)
JSI METAL= 0.22 (5) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
T.L. WISE
100083566
PROVINCE OF ONTARIO
March 10, 2017

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

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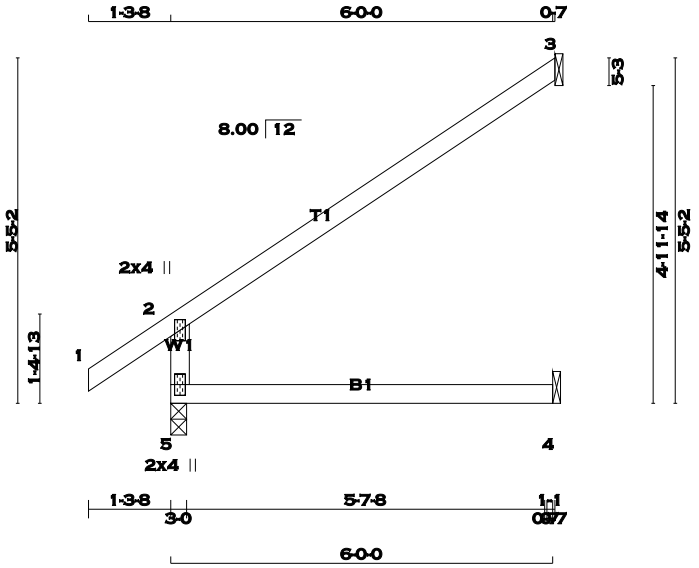

SCALE = 1:36.2

TOTAL WEIGHT = 18 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
5 - 2	2x4	DRY	No.2	SPF
1 - 3	2x4	DRY	No.2	SPF
5 - 4	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMV+p	MT20	2.0	4.0		
5	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
5	459	0	491	364	-177	3-0	3-0
3	175	0	209	0	-210	1-8	1-8
4	44	0	49	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) 3 , 4

PROVIDE ANCHORAGE AT BEARING JOINT 5 FOR 177 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 3 FOR 210 LBS FACTORED UPLIFT

PROVIDE FOR 364 LBS FACTORED HORIZONTAL REACTION AT JOINT 5

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
5	319	240 / 0	0 / 0	0 / 0	81 / -178	80 / 0	0 / 0
3	119	105 / 0	0 / 0	0 / 0	86 / -159	14 / 0	0 / 0
4	35	0 / 0	0 / 0	0 / 0	5 / -8	35 / 0	0 / 0
HORIZONTAL REACTIONS							
5	---	0 / 0	0 / 0	0 / 0	260 / -158	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 5

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
5- 2	-432 / 232	0.0	0.0	0.35 (7)	7.81			
1- 2	0 / 29	-77.3	-77.3	0.10 (1)	10.00			
2- 3	-126 / 16	-77.3	-77.3	0.39 (7)	6.25			
5- 4	0 / 0	-17.5	-17.5	0.13 (11)	10.00			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.39 (2-3:7) , BC=0.13 (4-5:11) , WB=0.00 (n/a:0) , SSI=0.25 (2-5:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.40 (2) (INPUT = 0.90)
JSI METAL= 0.24 (5) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
T.L. WISE
100083566
PROVINCE OF ONTARIO
March 10, 2017

KOTT

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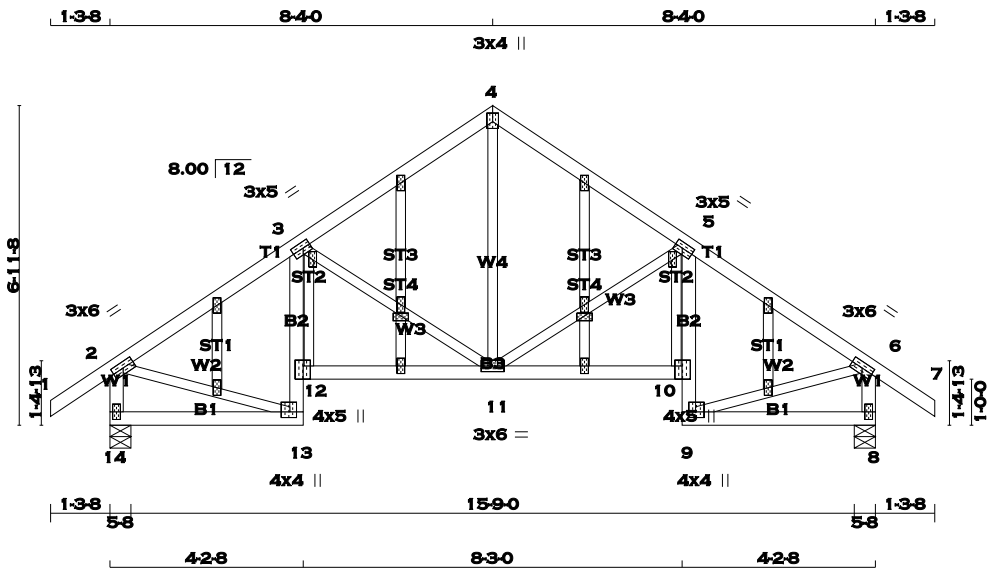

TOTAL WEIGHT = 88 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 7	2x4	DRY	No.2	SPF
14 - 2	2x4	DRY	No.2	SPF
8 - 6	2x4	DRY	No.2	SPF
14 - 13	2x4	DRY	No.2	SPF
13 - 3	2x4	DRY	No.2	SPF
12 - 10	2x4	DRY	No.2	SPF
9 - 5	2x4	DRY	No.2	SPF
9 - 8	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-t	MT20	3.0	6.0		
3	TMVW-t	MT20	3.0	5.0	1.50	2.00
3	NP+w	MT20	2.0	4.0	1.75	0.75
4	TTW+p	MT20	3.0	4.0	2.25	1.50
5	TMVW-t	MT20	3.0	5.0	1.50	2.00
5	NP+w	MT20	2.0	4.0	1.75	0.75
6	TMVW-t	MT20	3.0	6.0		
8	BMV1+p	MT20	2.0	4.0		
9	BMVW+p	MT20	4.0	4.0	1.50	1.75
10, 12, 18, 24						
10						
11	BMWWW-t	MT20	3.0	6.0		
12	BVMW+p	MT20	4.0	5.0		2.00
13	BMVW+p	MT20	4.0	4.0	1.50	1.75
14	BMV1+p	MT20	2.0	4.0		
15	NP-p	MT20	2.0	4.0	0.75	2.00
15, 16, 17, 19, 20, 21, 22, 23, 25, 26						
15	NP+w	MT20	2.0	4.0		
23	NP-p	MT20	2.0	4.0	0.75	2.00
24	BVMW+p	MT20	4.0	5.0	Edge	1.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

March 10, 2017

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
14	897	0	950	-316	-392	5-8	5-8
8	897	0	950	0	-392	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 14 FOR 392 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 8 FOR 392 LBS FACTORED UPLIFT

PROVIDE FOR 316 LBS FACTORED HORIZONTAL REACTION AT JOINT 14

ALLOW FOR 0.4" OF HORIZONTAL MOVEMENT DUE TO TOTAL LOAD

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
14	627	452 / 0	0 / 0	0 / 0	133 / -392	175 / 0	0 / 0
8	627	452 / 0	0 / 0	0 / 0	133 / -392	175 / 0	0 / 0

HORIZONTAL REACTIONS

14	---	0 / 0	0 / 0	0 / 0	226 / -226	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 14, 8

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 29	-77.3	-77.3	0.10 (1)	10.00	11- 4	-264 / 541	0.14 (7)
2- 3	-834 / 375	-77.3	-77.3	0.25 (7)	6.25	11- 5	-441 / 379	0.18 (4)
3- 4	-768 / 375	-77.3	-77.3	0.25 (7)	6.25	3-11	-457 / 408	0.19 (3)
4- 5	-783 / 408	-77.3	-77.3	0.26 (8)	6.25	2-13	-222 / 732	0.15 (1)
5- 6	-836 / 379	-77.3	-77.3	0.27 (8)	6.25	9- 6	-217 / 730	0.15 (1)
6- 7	0 / 29	-77.3	-77.3	0.10 (1)	10.00			
14- 2	-914 / 417	0.0	0.0	0.09 (1)	7.81			
8- 6	-914 / 417	0.0	0.0	0.09 (1)	7.81			
14-13	-295 / 306	-17.5	-17.5	0.08 (11)	6.25			
13-12	-153 / 83	0.0	0.0	0.90 (1)	7.81			
12- 3	-124 / 103	0.0	0.0	0.91 (1)	7.81			
12-11	-432 / 1035	-17.5	-17.5	0.19 (1)	6.25			
11-10	-256 / 958	-17.5	-17.5	0.19 (1)	6.25			
9-10	-152 / 81	0.0	0.0	0.90 (1)	7.81			
10- 5	-123 / 101	0.0	0.0	0.91 (1)	7.81			
9- 8	-10 / 21	-17.5	-17.5	0.08 (11)	10.00			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

KOTT

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DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.56")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.56")
CALCULATED VERT. DEFL.(TL) = L/ 815 (0.25")

CSI: TC=0.27 (5-6:8) , BC=0.91 (3-12:1) , WB=0.19 (3-11:3) , SSI=0.52 (12-13:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

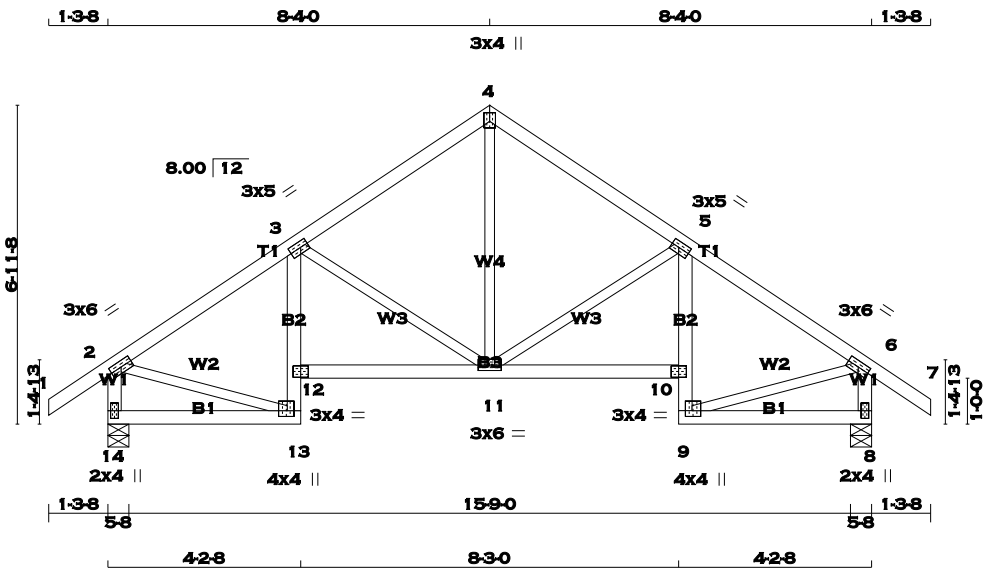
PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (13) (INPUT = 0.90)
JSI METAL= 0.25 (2) (INPUT = 1.00)

CONTINUED ON PAGE 2



TOTAL WEIGHT = 74 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 7	2x4	DRY	No.2	SPF
14 - 2	2x4	DRY	No.2	SPF
8 - 6	2x4	DRY	No.2	SPF
14 - 13	2x4	DRY	No.2	SPF
13 - 3	2x4	DRY	No.2	SPF
12 - 10	2x4	DRY	No.2	SPF
9 - 5	2x4	DRY	No.2	SPF
9 - 8	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-t	MT20	3.0	6.0		
3	TMVW-t	MT20	3.0	5.0	1.50	2.00
4	TTW+p	MT20	3.0	4.0	2.25	1.50
5	TMVW-t	MT20	3.0	5.0	1.50	2.00
6	TMVW-t	MT20	3.0	6.0		
8	BMV1+p	MT20	2.0	4.0		
9	BMVW+p	MT20	4.0	4.0	1.50	1.75
10	BVM-l	MT20	3.0	4.0		
11	BMWWW-t	MT20	3.0	6.0		
12	BVM-l	MT20	3.0	4.0		
13	BMVW+p	MT20	4.0	4.0	1.50	1.75
14	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
14	897	0	950	-316	-392	5-8	5-8
8	897	0	950	0	-392	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 14 FOR 392 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 8 FOR 392 LBS FACTORED UPLIFT

PROVIDE FOR 316 LBS FACTORED HORIZONTAL REACTION AT JOINT 14

ALLOW FOR 0.4" OF HORIZONTAL MOVEMENT DUE TO TOTAL LOAD

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
14	627	452 / 0	0 / 0	0 / 0	133 / -392	175 / 0	0 / 0
8	627	452 / 0	0 / 0	0 / 0	133 / -392	175 / 0	0 / 0
HORIZONTAL REACTIONS							
14	---	0 / 0	0 / 0	0 / 0	226 / -226	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 14, 8

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 29	-77.3	-77.3	0.10 (1)	10.00	11- 4	-264 / 541	0.14 (7)
2- 3	-834 / 375	-77.3	-77.3	0.25 (7)	6.25	11- 5	-441 / 379	0.18 (4)
3- 4	-768 / 375	-77.3	-77.3	0.25 (7)	6.25	3-11	-457 / 408	0.19 (3)
4- 5	-783 / 408	-77.3	-77.3	0.26 (8)	6.25	2-13	-222 / 732	0.15 (1)
5- 6	-836 / 379	-77.3	-77.3	0.27 (8)	6.25	9- 6	-217 / 730	0.15 (1)
6- 7	0 / 29	-77.3	-77.3	0.10 (1)	10.00			
14- 2	-914 / 417	0.0	0.0	0.09 (1)	7.81			
8- 6	-914 / 417	0.0	0.0	0.09 (1)	7.81			
14-13	-295 / 306	-17.5	-17.5	0.08 (11)	6.25			
13-12	-153 / 83	0.0	0.0	0.90 (1)	7.81			
12- 3	-124 / 103	0.0	0.0	0.91 (1)	7.81			
12-11	-432 / 1035	-17.5	-17.5	0.19 (1)	6.25			
11-10	-256 / 958	-17.5	-17.5	0.19 (1)	6.25			
9-10	-152 / 81	0.0	0.0	0.90 (1)	7.81			
10- 5	-123 / 101	0.0	0.0	0.91 (1)	7.81			
9- 8	-10 / 21	-17.5	-17.5	0.08 (11)	10.00			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	23.3	PSF
		DL =	3.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.0	PSF
TOTAL	LOAD	=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.56")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.56")
CALCULATED VERT. DEFL.(TL) = L/ 815 (0.25")

CSI: TC=0.27 (5-6:8) , BC=0.91 (3-12:1) , WB=0.19 (3-11:3) , SSI=0.52 (12-13:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (13) (INPUT = 0.90)
JSI METAL= 0.25 (12) (INPUT = 1.00)

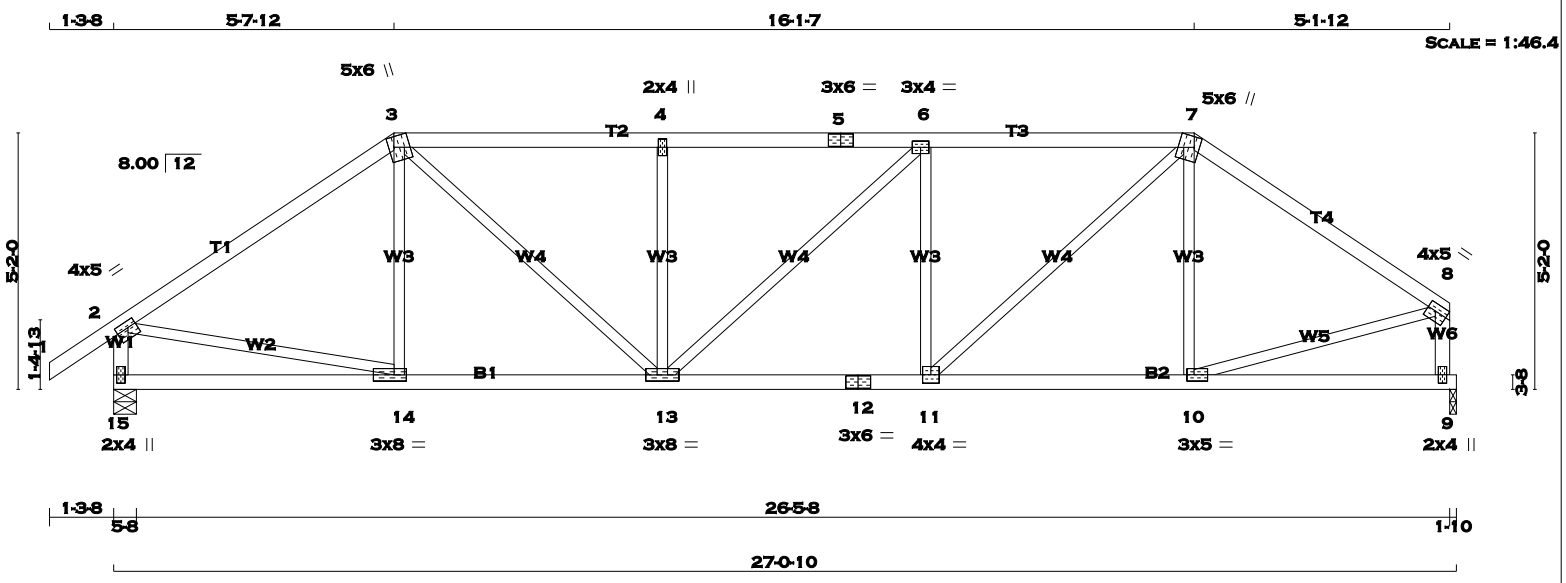
LICENSED PROFESSIONAL ENGINEER
T.L. WISE
100083566
PROVINCE OF ONTARIO

March 10, 2017

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

RECEIVED
TOWN OF MILTON
MAR 29, 2017
17-4978
BUILDING DIVISION



TOTAL WEIGHT = 2 X 109 = 218 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 3	2x4	DRY	No.2	SPF
3 - 5	2x4	DRY	No.2	SPF
5 - 7	2x4	DRY	No.2	SPF
7 - 8	2x4	DRY	No.2	SPF
15 - 2	2x4	DRY	No.2	SPF
9 - 8	2x4	DRY	No.2	SPF
15 - 12	2x4	DRY	No.2	SPF
12 - 9	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-t	MT20	4.0	5.0	1.50	2.00
3	TTWW+m	MT20	5.0	6.0	Edge	3.75
4	TMW+w	MT20	2.0	4.0		
5	TS-t	MT20	3.0	6.0		
6	TMWW-t	MT20	3.0	4.0		
7	TTWW+m	MT20	5.0	6.0	Edge	3.75
8	TMVW-t	MT20	4.0	5.0	1.75	Edge
9	BMV1+p	MT20	2.0	4.0		
10	BMWW-t	MT20	3.0	5.0	1.50	1.75
11	BMWW-t	MT20	4.0	4.0		
12	BS-t	MT20	3.0	6.0		
13	BMWWW-t	MT20	3.0	8.0		
14	BMWW-t	MT20	3.0	8.0	1.50	3.00
15	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

March 10, 2017

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
15	1383	0	1467	241	-712	5-8	5-8
9	1276	0	1318	0	-648	1-10	1-10

PROVIDE ANCHORAGE AT BEARING JOINT 15 FOR 712 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 9 FOR 648 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 241 LBS FACTORED HORIZONTAL REACTION AT JOINT 15

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
15	968	691 / 0	0 / 0	0 / 0	211 / -687	277 / 0	0 / 0
9	896	627 / 0	0 / 0	0 / 0	103 / -636	269 / 0	0 / 0
HORIZONTAL REACTIONS							
15	---	0 / 0	0 / 0	0 / 0	172 / -158	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 15, 9

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.63 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 29	-77.3	-77.3	0.10 (1)	10.00	14- 3	-118 / 165	0.04 (3)
2- 3	-1506 / 753	-77.3	-77.3	0.54 (7)	4.74	3-13	-466 / 783	0.45 (8)
3- 4	-1851 / 1042	-77.3	-77.3	0.36 (1)	4.64	13- 4	-471 / 364	0.18 (3)
4- 5	-1851 / 1043	-77.3	-77.3	0.37 (1)	4.63	13- 6	-65 / 56	0.06 (6)
5- 6	-1851 / 1043	-77.3	-77.3	0.37 (1)	4.63	11- 6	-508 / 403	0.19 (3)
6- 7	-1814 / 1021	-77.3	-77.3	0.37 (1)	4.67	11- 7	-505 / 895	0.49 (7)
7- 8	-1389 / 708	-77.3	-77.3	0.46 (8)	5.10	10- 7	-206 / 197	0.08 (3)
15- 2	-1426 / 741	0.0	0.0	0.14 (1)	6.88	2-14	-467 / 1241	0.27 (1)
9- 8	-1281 / 674	0.0	0.0	0.13 (1)	7.17	10- 8	-474 / 1196	0.26 (1)
15-14	-220 / 212	-17.5	-17.5	0.13 (11)	6.25			
14-13	-552 / 1272	-17.5	-17.5	0.26 (1)	6.25			
13-12	-801 / 1816	-17.5	-17.5	0.32 (1)	6.25			
12-11	-801 / 1816	-17.5	-17.5	0.32 (1)	6.25			
11-10	-426 / 1151	-17.5	-17.5	0.24 (1)	6.25			
10- 9	-13 / 28	-17.5	-17.5	0.12 (11)	6.25			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.90")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.90")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.54 (2-3:7) , BC=0.32 (11-13:1) , WB=0.49 (7-11:7) , SSI=0.19 (6-7:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

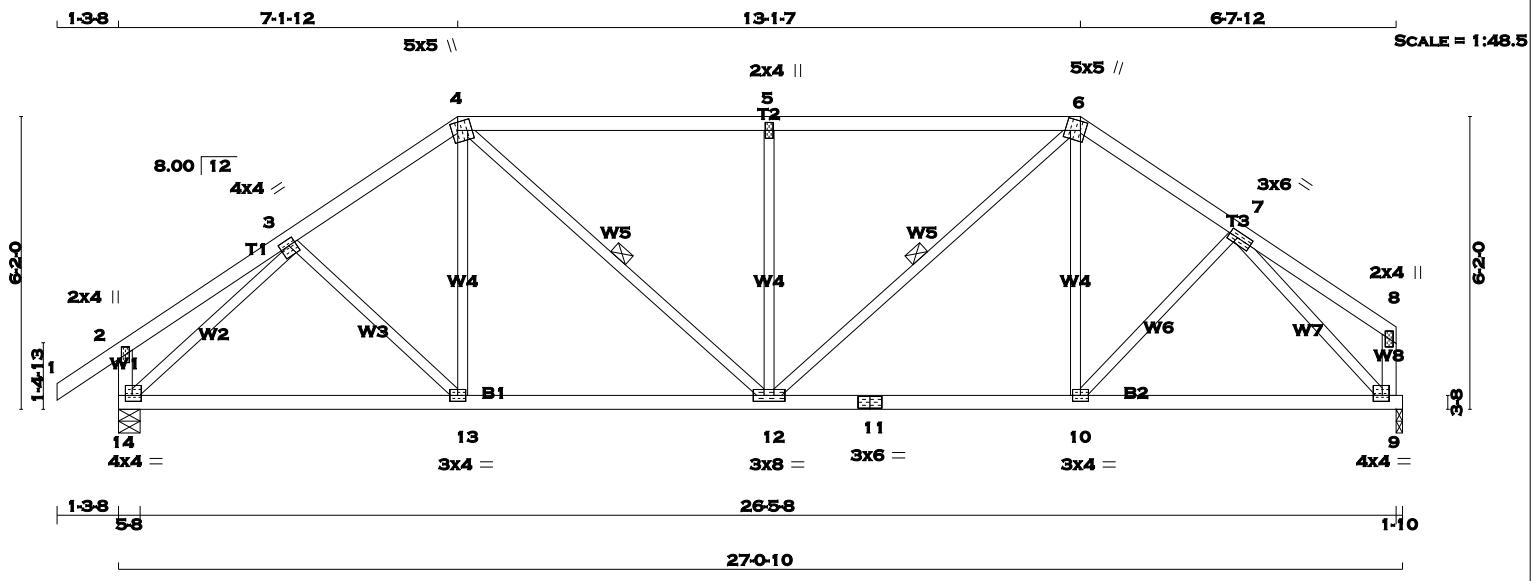
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (10) (INPUT = 0.90)
JSI METAL= 0.51 (2) (INPUT = 1.00)



TOTAL WEIGHT = 2 X 112 = 224 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 6	2x4	DRY	No.2	SPF
6 - 8	2x4	DRY	No.2	SPF
14 - 2	2x4	DRY	No.2	SPF
9 - 8	2x4	DRY	No.2	SPF
14 - 11	2x4	DRY	No.2	SPF
11 - 9	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMV+p	MT20	2.0	4.0		
3	TMWW-t	MT20	4.0	4.0	1.50	1.75
4	TTWW+m	MT20	5.0	5.0	2.00	1.50
5	TMW+w	MT20	2.0	4.0		
6	TTWW+m	MT20	5.0	5.0	2.25	1.25
7	TMWW-t	MT20	3.0	6.0	1.50	2.25
8	TMV+p	MT20	2.0	4.0		
9	BMVW1-t	MT20	4.0	4.0	1.50	1.75
10	BMWW-t	MT20	3.0	4.0		
11	BS-t	MT20	3.0	6.0		
12	BMWWW-t	MT20	3.0	8.0		
13	BMWW-t	MT20	3.0	4.0		
14	BMVW1-t	MT20	4.0	4.0	1.50	1.75

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
14	1383	0	1471	281	-698	5-8	5-8
9	1276	0	1319	0	-634	1-10	1-10

PROVIDE ANCHORAGE AT BEARING JOINT 14 FOR 698 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 9 FOR 634 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 281 LBS FACTORED HORIZONTAL REACTION AT JOINT 14

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
14	968	691 / 0	0 / 0	0 / 0	220 / -677	277 / 0	0 / 0
9	896	627 / 0	0 / 0	0 / 0	106 / -626	269 / 0	0 / 0
HORIZONTAL REACTIONS							
14	---	0 / 0	0 / 0	0 / 0	200 / -186	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 14, 9

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.76 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 4-12, 6-12. DBS = 20-0-0 . CBF = 44 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 29	-77.3	-77.3	0.10	(1)	10.00	3-13	-35 / 219	0.04 (7)
2- 3	-25 / 159	-77.3	-77.3	0.18	(7)	6.25	13- 4	-50 / 163	0.04 (11)
3- 4	-1502 / 822	-77.3	-77.3	0.28	(7)	5.27	4-12	-366 / 522	0.18 (8)
4- 5	-1622 / 933	-77.3	-77.3	0.52	(7)	4.76	12- 5	-656 / 507	0.38 (3)
5- 6	-1622 / 933	-77.3	-77.3	0.52	(7)	4.76	12- 6	-402 / 631	0.20 (7)
6- 7	-1401 / 787	-77.3	-77.3	0.25	(8)	5.46	10- 6	-22 / 143	0.03 (11)
7- 8	-24 / 104	-77.3	-77.3	0.15	(8)	6.25	10- 7	-65 / 162	0.03 (5)
14- 2	-249 / 237	0.0	0.0	0.04	(7)	7.81	14- 3	-1681 / 714	0.63 (1)
9- 8	-129 / 137	0.0	0.0	0.03	(8)	7.81	7- 9	-1584 / 719	0.58 (1)
14-13	-665 / 1273	-17.5	-17.5	0.30	(1)	6.25			
13-12	-506 / 1250	-17.5	-17.5	0.30	(1)	6.25			
12-11	-391 / 1155	-17.5	-17.5	0.29	(1)	6.25			
11-10	-391 / 1155	-17.5	-17.5	0.29	(1)	6.25			
10- 9	-45[illegible] / 10[illegible]	[illegible]	[illegible]			6.25			

WIND LOAD APPLIED TO THIS TRUSS HAS BEEN CALCULATED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM/TERTIARY} WIN[illegible] MAY BE LOCATED ON {OPE[illegible] LEAST {1-0-0} FT IN-SX AWAY FRO[illegible]

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.90")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.90")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.52 (4-5:7) , BC=0.30 (12-13:1) , WB=0.63 (3-14:1) , SSI=0.25 (4-5:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

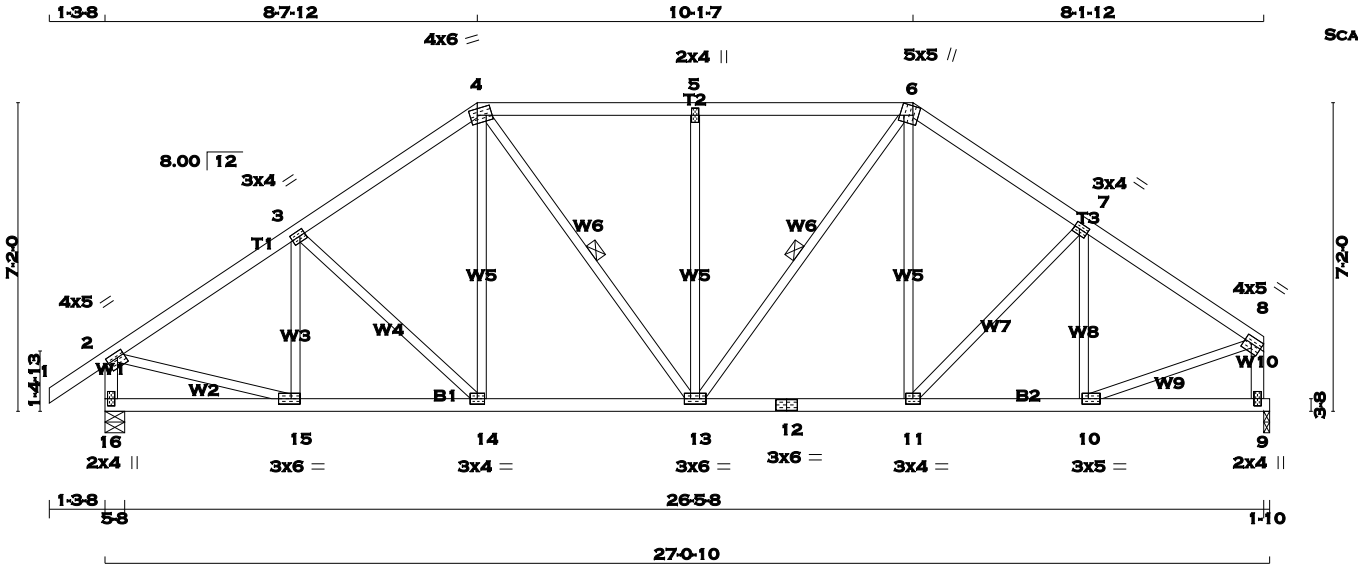
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (14) (INPUT = 0.90)
JSI METAL= 0.62 (3) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
T.L. WISE
100083566
PROVINCE OF ONTARIO
March 10, 2017

KOTT

RECEIVED
TOWN OF WHITCHURCH-STOUFFVILLE
MAR 20, 2017
17-4978
BUILDING DIVISION



TOTAL WEIGHT = 2 X 121 = 243 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 6	2x4	DRY	No.2	SPF
6 - 8	2x4	DRY	No.2	SPF
16 - 2	2x4	DRY	No.2	SPF
9 - 8	2x4	DRY	No.2	SPF
16 - 12	2x4	DRY	No.2	SPF
12 - 9	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-t	MT20	4.0	5.0	1.50	2.00
3	TMWW-t	MT20	3.0	4.0	1.50	1.50
4	TTWW-m	MT20	4.0	6.0	1.75	2.00
5	TMW+w	MT20	2.0	4.0		
6	TTWW+m	MT20	5.0	5.0	2.50	1.50
7	TMWW-t	MT20	3.0	4.0	1.50	1.50
8	TMVW-t	MT20	4.0	5.0	1.75	Edge
9	BMV1+p	MT20	2.0	4.0		
10	BMWW-t	MT20	3.0	5.0	1.50	1.75
11	BMWW-t	MT20	3.0	4.0		
12	BS-t	MT20	3.0	6.0		
13	BMWWW-t	MT20	3.0	6.0		
14	BMWW-t	MT20	3.0	4.0		
15	BMWW-t	MT20	3.0	6.0	1.50	2.50
16	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
16	1383	0	1473	320	-681	5-8	5-8
9	1276	0	1330	0	-616	1-10	1-10

PROVIDE ANCHORAGE AT BEARING JOINT 16 FOR 681 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 9 FOR 616 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 320 LBS FACTORED HORIZONTAL REACTION AT JOINT 16

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
16	968	691 / 0	0 / 0	0 / 0	225 / -665	277 / 0	0 / 0
9	896	627 / 0	0 / 0	0 / 0	133 / -613	269 / 0	0 / 0
HORIZONTAL REACTIONS							
16	---	0 / 0	0 / 0	0 / 0	229 / -214	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 16, 9

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.15 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 4-13, 6-13. DBS = 20-0-0 . CBF = 31 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
1- 2	0 / 29	-77.3	-77.3	0.10 (1)		10.00
2- 3	-1539 / 749	-77.3	-77.3	0.35 (7)		5.15
3- 4	-1430 / 800	-77.3	-77.3	0.36 (7)		5.29
4- 5	-1365 / 807	-77.3	-77.3	0.34 (7)		5.33
5- 6	-1365 / 807	-77.3	-77.3	0.34 (7)		5.33
6- 7	-1357 / 777	-77.3	-77.3	0.33 (8)		5.44
7- 8	-1370 / 682	-77.3	-77.3	0.32 (8)		5.40
16- 2	-1438 / 702	0.0	0.0	0.14 (1)		6.85
9- 8	-1297 / 636	0.0	0.0	0.13 (1)		7.12
16-15	-299 / 290	-17.5	-17.5	0.07 (11)		6.25
15-14	-654 / 1350	-17.5	-17.5	0.24 (1)		6.25
14-13	-456 / 1189	-17.5	-17.5	0.22 (1)		6.25
13-12	-355 / 1115	-17.5	-17.5	0.21 (1)		6.25
12-11	-355 / 1115	-17.5	-17.5	0.21 (1)		6.25
11-10	-456 / 1189	-17.5	-17.5	0.22 (1)		6.25
10- 9	-654 / 1350	-17.5	-17.5	0.24 (1)		6.25

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
15- 3	-233 / 182	0.06 (1)
3-14	-226 / 273	0.13 (3)
14- 4	-134 / 238	0.12 (7)
4-13	-254 / 345	0.12 (8)
13- 5	-502 / 388	0.44 (3)
13- 6	-284 / 427	0.13 (7)
11- 6	-103 / 178	0.09 (8)
10- 7	-317 / 219	0.09 (1)
2-15	-489 / 1311	0.28 (1)
10- 8	-491 / 1219	0.26 (1)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF {40-0
COE EXTERNAL PEAK
WIN STING SYSTEM) INTERNAL
{OPE MAY BE LOCATED ON
FRO LEAST {0-0} FT IN SX AWAY

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.90")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.90")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.36 (3-4:7) , BC=0.24 (14-15:1) , WB=0.44 (5-13:3) , SSI=0.19 (4-5:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

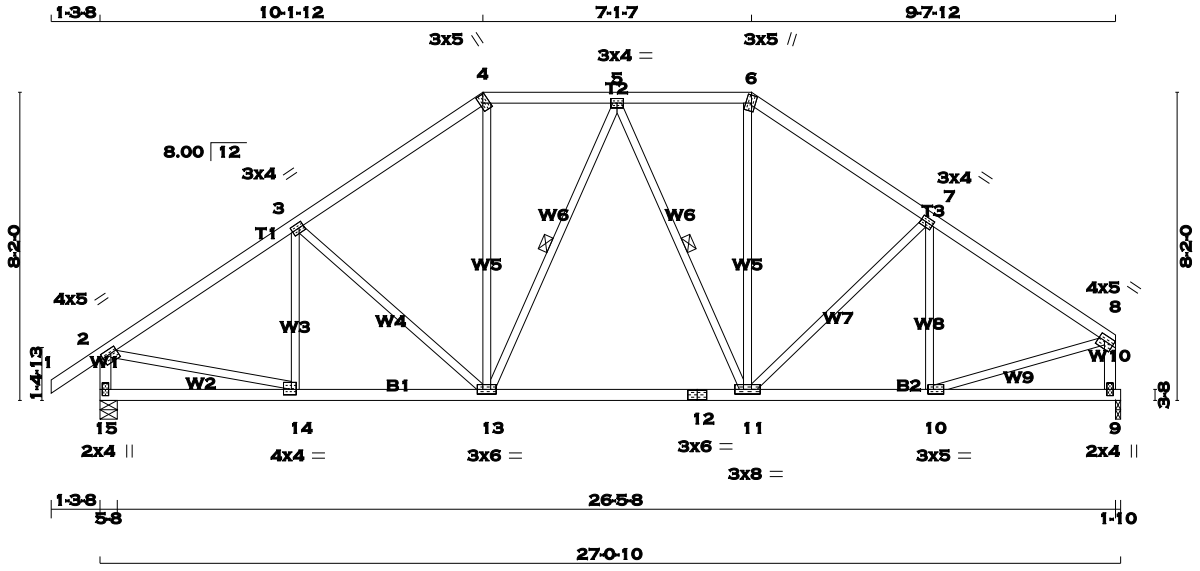
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (10) (INPUT = 0.90)
JSI METAL= 0.51 (2) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
T.L. WISE
100083566
PROVINCE OF ONTARIO
March 10, 2017

KOTT

RECEIVED
TOWN OF MILTON
MAR 17 2017
BUILDING DIVISION



TOTAL WEIGHT = 2 X 122 = 244 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 6	2x4	DRY	No.2	SPF
6 - 8	2x4	DRY	No.2	SPF
15 - 2	2x4	DRY	No.2	SPF
9 - 8	2x4	DRY	No.2	SPF
15 - 12	2x4	DRY	No.2	SPF
12 - 9	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-t	MT20	4.0	5.0	1.50	2.00
3	TMWW-t	MT20	3.0	4.0	1.50	1.50
4	TTW+h	MT20	3.0	5.0	2.50	1.00
5	TMWW-t	MT20	3.0	4.0		
6	TTW+m	MT20	3.0	5.0	2.50	1.25
7	TMWW-t	MT20	3.0	4.0	1.50	1.50
8	TMVW-t	MT20	4.0	5.0	1.75	Edge
9	BMV1+p	MT20	2.0	4.0		
10	BMWW-t	MT20	3.0	5.0	1.50	1.75
11	BMWWW-t	MT20	3.0	8.0		
12	BS-t	MT20	3.0	6.0		
13	BMWWW-t	MT20	3.0	6.0		
14	BMWW-t	MT20	4.0	4.0	1.75	1.50
15	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
15	1383	0	1474	359	-661	5-8	5-8
9	1276	0	1339	0	-595	1-10	1-10

PROVIDE ANCHORAGE AT BEARING JOINT 15 FOR 661 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 9 FOR 595 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 359 LBS FACTORED HORIZONTAL REACTION AT JOINT 15

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
15	968	691 / 0	0 / 0	0 / 0	228 / -650	277 / 0	0 / 0
9	896	627 / 0	0 / 0	0 / 0	156 / -598	269 / 0	0 / 0
HORIZONTAL REACTIONS							
15	---	0 / 0	0 / 0	0 / 0	257 / -242	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 15, 9

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.02 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 5-13, 5-11. DBS = 20-0-0 . CBF = 27 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
1- 2	0 / 29	-77.3	-77.3	0.10 (1)		10.00
2- 3	-1557 / 731	-77.3	-77.3	0.45 (7)		5.02
3- 4	-1352 / 746	-77.3	-77.3	0.45 (7)		5.31
4- 5	-1121 / 709	-77.3	-77.3	0.21 (7)		5.91
5- 6	-1072 / 693	-77.3	-77.3	0.20 (8)		6.01
6- 7	-1306 / 732	-77.3	-77.3	0.41 (8)		5.43
7- 8	-1415 / 675	-77.3	-77.3	0.41 (8)		5.25
15- 2	-1434 / 686	0.0	0.0	0.14 (1)		6.86
9- 8	-1301 / 619	0.0	0.0	0.13 (1)		7.12
15-14	-338 / 329	-17.5	-17.5	0.10 (11)		6.25
14-13	-653 / 1379	-17.5	-17.5	0.28 (1)		6.25
13-12	-403 / 1175	-17.5	-17.5	0.25 (1)		6.25
12-11	-403 / 1175	-17.5	-17.5	0.25 (1)		6.25
11-10	-41 / 1	-17	-17	0.0		6.25
10- 9	/ 2	-17	-17	0.0 (11)		6.25

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
14- 3	-180 / 164	0.06 (3)
13-13	-348 / 365	0.29 (3)
13- 4	-232 / 456	0.30 (8)
13- 5	-185 / 218	0.09 (4)
5-11	-252 / 244	0.12 (3)
11- 6	-234 / 459	0.30 (7)
11- 7	-259 / 311	0.21 (4)
10- 7	-257 / 194	0.09 (3)
2-14	-457 / 1319	0.28 (1)
10- 8	-465 / 1239	0.27 (1)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF {0.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COE... STING SYSTEM}.INTERNAL WIN... G MAY BE APPLIED IN {OPE... LEAST {0-0} FT-IN-SX AWAY FRO...

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.90")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.90")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.45 (3-4:7) , BC=0.28 (13-14:1) , WB=0.30 (6-11:7) , SSI=0.17 (2-3:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (6) (INPUT = 0.90)
JSI METAL= 0.52 (2) (INPUT = 1.00)

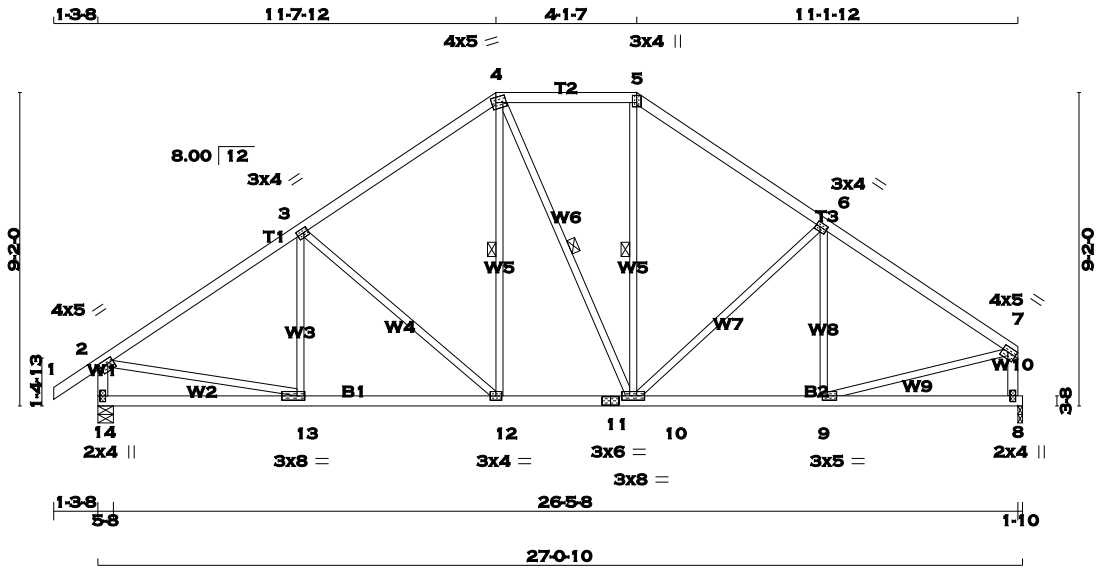
LICENSED PROFESSIONAL ENGINEER
T.L. WISE
100083566
PROVINCE OF ONTARIO

March 10, 2017

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

KOTT

RECEIVED
TOWN OF MILTON
MAR 29 2017
17-4978
BUILDING DIVISION



SCALE = 1:67.4

TOTAL WEIGHT = 2 X 122 = 244 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 5	2x4	DRY	No.2	SPF
5 - 7	2x4	DRY	No.2	SPF
14 - 2	2x4	DRY	No.2	SPF
8 - 7	2x4	DRY	No.2	SPF
14 - 11	2x4	DRY	No.2	SPF
11 - 8	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-t	MT20	4.0	5.0	1.50	2.00
3	TMWW-t	MT20	3.0	4.0	1.50	1.50
4	TTWW-m	MT20	4.0	5.0	1.75	1.50
5	TTW+p	MT20	3.0	4.0	2.50	1.50
6	TMWW-t	MT20	3.0	4.0	1.50	1.50
7	TMVW-t	MT20	4.0	5.0	1.75	Edge
8	BMV1+p	MT20	2.0	4.0		
9	BMWW-t	MT20	3.0	5.0	1.50	1.75
10	BMWWW-t	MT20	3.0	8.0		
11	BS-t	MT20	3.0	6.0		
12	BMWW-t	MT20	3.0	4.0		
13	BMWW-t	MT20	3.0	8.0	1.50	2.75
14	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
14	1383	0	1473	398	-638	5-8	5-8
8	1276	0	1347	0	-571	1-10	1-10

PROVIDE ANCHORAGE AT BEARING JOINT 14 FOR 638 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 8 FOR 571 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 398 LBS FACTORED HORIZONTAL REACTION AT JOINT 14

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
14	968	691 / 0	0 / 0	0 / 0	226 / -634	277 / 0	0 / 0
8	896	627 / 0	0 / 0	0 / 0	177 / -581	269 / 0	0 / 0
HORIZONTAL REACTIONS							
14	---	0 / 0	0 / 0	0 / 0	285 / -270	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 14, 8

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 4-12, 4-10, 5-10. DBS = 20-0-0 . CBF = 25 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 29	-77.3	-77.3	1	0.10 (1)	10.00	13- 3	-120 / 163	0.05 (3)
2- 3	-1563 / 698	-77.3	-77.3	1	0.56 (7)	4.89	3-12	-476 / 443	0.56 (3)
3- 4	-1252 / 687	-77.3	-77.3	1	0.55 (7)	5.34	12- 4	-232 / 391	0.12 (7)
4- 5	-1007 / 662	-77.3	-77.3	1	0.25 (8)	6.09	4-10	-166 / 169	0.11 (5)
5- 6	-1234 / 681	-77.3	-77.3	1	0.52 (8)	5.44	10- 5	-174 / 345	0.09 (7)
6- 7	-1444 / 650	-77.3	-77.3	1	0.51 (8)	5.09	10- 6	-389 / 393	0.45 (4)
14- 2	-1430 / 666	0.0	0.0	1	0.14 (1)	6.87	9- 6	-193 / 188	0.09 (3)
8- 7	-1306 / 598	0.0	0.0	1	0.13 (1)	7.11	2-13	-414 / 1318	0.29 (1)
							9- 7	-426 / 1254	0.27 (1)
14-13	-377 / 369	-17.5	-17.5	1	0.15 (11)	6.25			
13-12	-638 / 1396	-17.5	-17.5	1	0.28 (1)	6.25			
12-11	-306 / 1040	-17.5	-17.5	1	0.20 (1)	6.25			
11-10	-306 / 1040	-17.5	-17.5	1	0.20 (1)	6.25			
10- 9	-386 / 1217	-17.5	-17.5	1	0.25 (1)	6.25			
9- 8	-1 / 25	-1	0	1	0.1 (1)	6.25			

WIND LOADS APPLIED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} MEAN HT IN-SX, REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM/TERTIARY} WIN G MAY BE LOCATED ON {OPE LEAST {1-0-0} FROM

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.90")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.90")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.56 (2-3:7) , BC=0.28 (12-13:1) , WB=0.56 (3-12:3) , SSI=0.19 (2-3:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

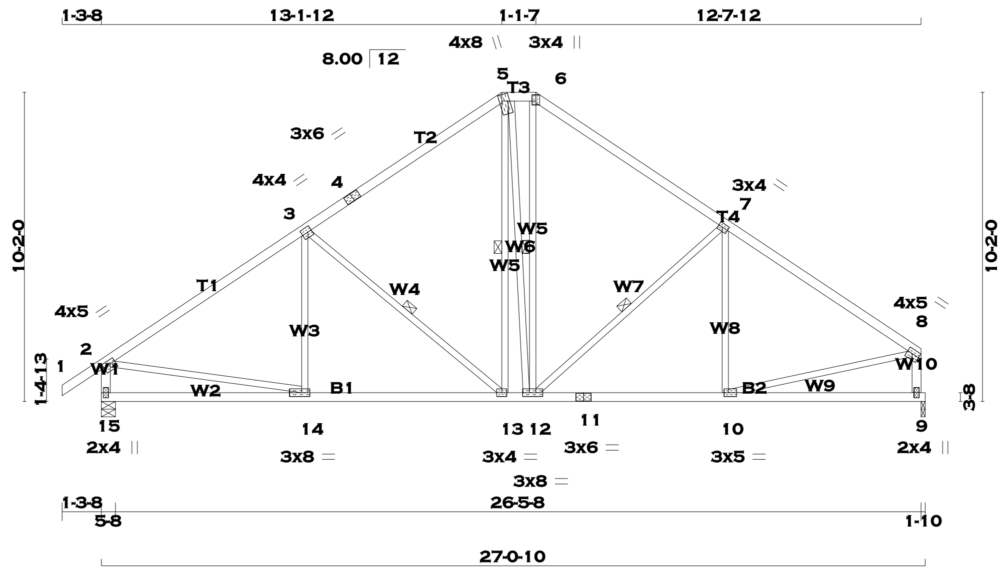
JSI GRIP= 0.90 (10) (INPUT = 0.90)
JSI METAL= 0.52 (2) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
T.L. WISE
100083566
PROVINCE OF ONTARIO
March 10, 2017

KOTT

RECEIVED
TOWN OF MILTON
MAR 20, 2017
17-4978
BUILDING DIVISION

ID:4BORDhR74Ei?Dog_xdfUkJyKZi_-WfhHtIZWuuL_Ov3h6LB1j2n4I6ypQ83j2Alu3_zcJJt



TOTAL WEIGHT = 2 X 128 = 256 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 5	2x4	DRY	No.2	SPF
5 - 6	2x4	DRY	No.2	SPF
6 - 8	2x4	DRY	No.2	SPF
15 - 2	2x4	DRY	No.2	SPF
9 - 8	2x4	DRY	No.2	SPF
15 - 11	2x4	DRY	No.2	SPF
11 - 9	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-t	MT20	4.0	5.0	1.50	2.00
3	TMWW-t	MT20	4.0	4.0	2.00	1.50
4	TS-t	MT20	3.0	6.0		
5	TTWW+m	MT20	4.0	8.0	2.50	1.00
6	TTW+p	MT20	3.0	4.0	2.50	1.50
7	TMWW-t	MT20	3.0	4.0	1.50	1.50
8	TMVW-t	MT20	4.0	5.0	1.75	Edge
9	BMV1+p	MT20	2.0	4.0		
10	BMWW-t	MT20	3.0	5.0	1.50	1.75
11	BS-t	MT20	3.0	6.0		
12	BMWWW-t	MT20	3.0	8.0		
13	BMWW-t	MT20	3.0	4.0		
14	BMWW-t	MT20	3.0	8.0	1.50	3.25
15	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
15	1383	0	1471	438	-611	5-8	5-8
9	1276	0	1354	0	-544	1-10	1-10

PROVIDE ANCHORAGE AT BEARING JOINT 15 FOR 611 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 9 FOR 544 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 438 LBS FACTORED HORIZONTAL REACTION AT JOINT 15

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
		MAX./MIN. COMPONENT REACTIONS					
15	968	691 / 0	0 / 0	0 / 0	222 / -615	277 / 0	0 / 0
9	896	627 / 0	0 / 0	0 / 0	193 / -562	269 / 0	0 / 0
HORIZONTAL REACTIONS							
15	---	0 / 0	0 / 0	0 / 0	313 / -298	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 15, 9

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.72 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 3-13, 5-13, 5-12, 6-12, 7-12. DBS = 20-0-0 . CBF = 65 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (11)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
1- 2	0 / 29	-77.3	-77.3	0.10 (1)	10.00
2- 3	-1554 / 656	-77.3	-77.3	0.68 (7)	4.72
3- 4	-1156 / 646	-77.3	-77.3	0.67 (7)	5.35
4- 5	-1156 / 646	-77.3	-77.3	0.67 (7)	5.35
5- 6	-941 / 620	-77.3	-77.3	0.10 (8)	6.25
6- 7	-1162 / 655	-77.3	-77.3	0.63 (8)	5.43
7- 8	-1456 / 615	-77.3	-77.3	0.63 (8)	4.91
15- 2	-1424 / 644	0.0	0.0	0.14 (1)	6.88
9- 8	-1307 / 575	0.0	0.0	0.13 (1)	7.11
15-14	-416 / 408	-17.5	-17.5	0.19 (11)	6.25
14-13	-617 / 1401	-17.5	-17.5	0.30 (1)	6.25
13-12	-215 / 951	-17.5	-17.5	0.22 (1)	6.25
12-11	-362 / 1231	-17.5	-17.5	0.27 (1)	6.25
11-10	-36[illegible] / 1[illegible]	[illegible]	[illegible]	[illegible]	6.25
10- 9	[illegible] / 2[illegible]	[illegible]	[illegible]5	0.1[illegible] (11)	6.25

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
14- 3	-78 / 161	0.04 (4)
3-13	-594 / 528	0.29 (3)
13- 5	-255 / 397	0.18 (7)
5-12	-170 / 164	0.12 (5)
12- 6	-246 / 404	0.17 (7)
12- 7	-503 / 478	0.24 (4)
10- 7	-150 / 185	0.08 (3)
2-14	-393 / 1325	0.31 (8)
10- 8	-399 / 1261	0.30 (7)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF {0.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COE[illegible] SING SYSTEM}.INTERNAL WIN[illegible] G MAY BE LOCATED {0} {OPE[illegible] LEAST {0-0} FT-IN-SX AWAY FRO[illegible]

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.90")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.90")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.68 (2-3:7) , BC=0.30 (13-14:1) , WB=0.31 (2-14:8) , SSI=0.22 (2-3:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

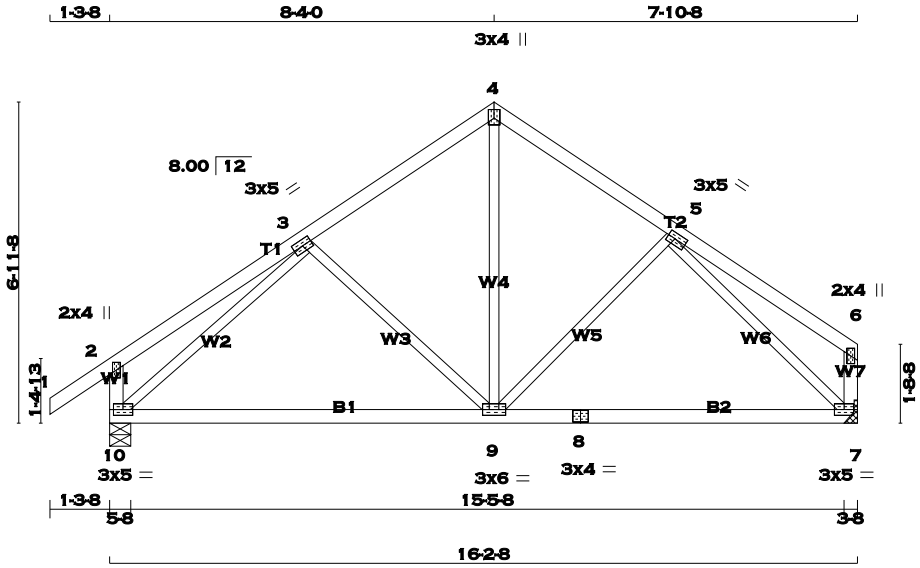
PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (10) (INPUT = 0.90)
JSI METAL= 0.53 (2) (INPUT = 1.00)

March 10, 2017



TOTAL WEIGHT = 3 X 68 = 203 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 6	2x4	DRY	No.2	SPF
10 - 2	2x4	DRY	No.2	SPF
7 - 6	2x4	DRY	No.2	SPF
10 - 8	2x4	DRY	No.2	SPF
8 - 7	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMV+p	MT20	2.0	4.0		
3	TMWW-t	MT20	3.0	5.0		
4	TTW+p	MT20	3.0	4.0	2.25	1.50
5	TMWW-t	MT20	3.0	5.0	1.50	2.00
6	TMV+p	MT20	2.0	4.0		
7	BMVW1-t	MT20	3.0	5.0		
8	BS-t	MT20	3.0	4.0		
9	BMWWW-t	MT20	3.0	6.0		
10	BMVW1-t	MT20	3.0	5.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
10	875	0	928	311	-382	5-8	5-8
7	769	0	813	0	-317	HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-8	

PROVIDE ANCHORAGE AT BEARING JOINT 10 FOR 382 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 7 FOR 317 LBS FACTORED UPLIFT

PROVIDE FOR 311 LBS FACTORED HORIZONTAL REACTION AT JOINT 10

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
10	612	441 / 0	0 / 0	0 / 0	132 / -382	170 / 0	0 / 0
7	539	377 / 0	0 / 0	0 / 0	112 / -331	162 / 0	0 / 0
HORIZONTAL REACTIONS							
10	---	0 / 0	0 / 0	0 / 0	222 / -208	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 10

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 29	-77.3	-77.3	0.10 (1)	10.00	3- 9	-262 / 372	0.14 (3)
2- 3	-26 / 170	-77.3	-77.3	0.25 (7)	6.25	9- 4	-239 / 443	0.19 (7)
3- 4	-658 / 363	-77.3	-77.3	0.28 (7)	6.25	9- 5	-200 / 327	0.10 (4)
4- 5	-658 / 371	-77.3	-77.3	0.25 (8)	6.25	10- 3	-922 / 292	0.47 (4)
5- 6	-24 / 115	-77.3	-77.3	0.22 (8)	6.25	5- 7	-890 / 312	0.44 (3)
10- 2	-267 / 257	0.0	0.0	0.04 (7)	7.81			
7- 6	-147 / 156	0.0	0.0	0.03 (8)	7.81			
10- 9	-351 / 727	-17.5	-17.5	0.37 (11)	6.25			
9- 8	-191 / 623	-17.5	-17.5	0.37 (11)	6.25			
8- 7	-191 / 623	-17.5	-17.5	0.37 (11)	6.25			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.54")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.54")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.28 (3-4:7) , BC=0.37 (9-10:11) , WB=0.47 (3-10:4) , SSI=0.14 (3-4:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (3) (INPUT = 0.90)
JSI METAL= 0.26 (3) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
T.L. WISE
100083566
PROVINCE OF ONTARIO
March 10, 2017

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

RECEIVED
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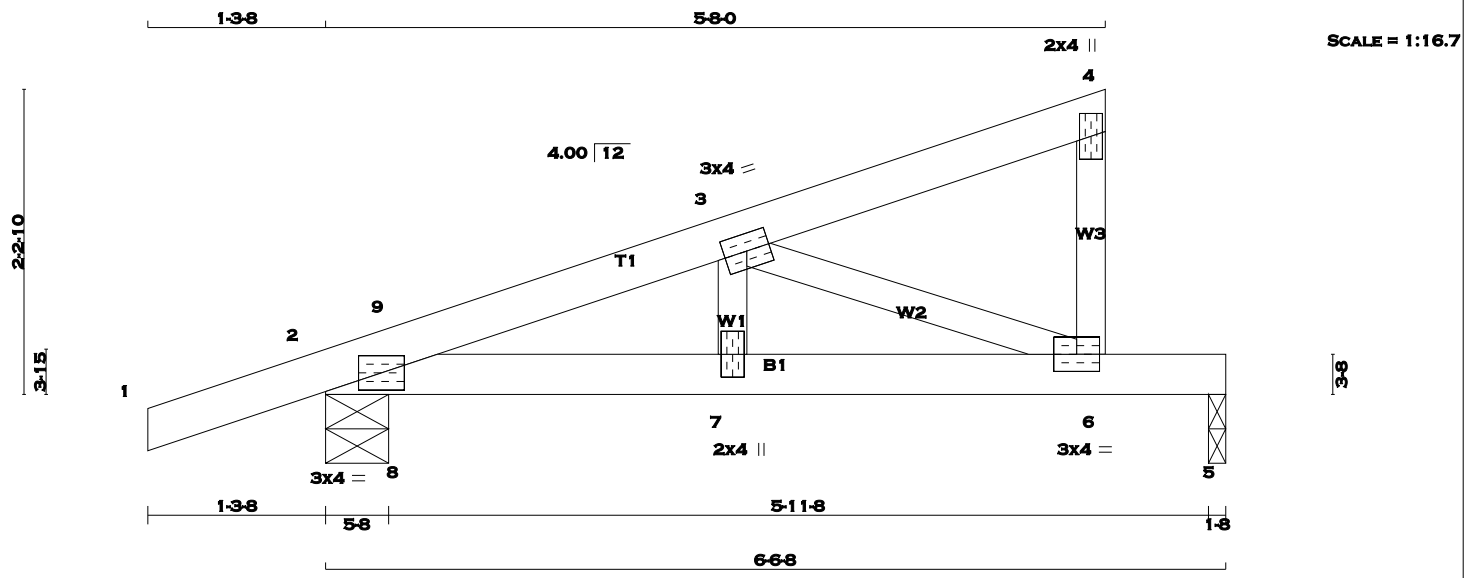


TOTAL WEIGHT = 18 X 20 = 358 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
6 - 4	2x3	DRY	No.2	SPF
2 - 5	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMB1-I	MT20	3.0	4.0		
3	TMWW-t	MT20	3.0	4.0		
4	TMV+p	MT20	2.0	4.0		
6	BMVW-t	MT20	3.0	4.0		
7	BMW+w	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
2	409	0	409	151	-217	5-8	5-8
5	247	0	247	0	-121	1-8	1-8

PROVIDE ANCHORAGE AT BEARING JOINT 2 FOR 217 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 5 FOR 150 LBS FACTORED UPLIFT

PROVIDE FOR 151 LBS FACTORED HORIZONTAL REACTION AT JOINT 2

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
2	285	212 / 0	0 / 0	0 / 0	0 / -202	73 / 0	0 / 0
5	175	114 / 0	0 / 0	0 / 0	0 / -125	61 / 0	0 / 0
HORIZONTAL REACTIONS							
2	---	0 / 0	0 / 0	0 / 0	108 / 0	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 2, 5

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 15	-77.3	-77.3	0.10 (1)	10.00	7- 3	-30 / 178	0.04 (1)
2- 9	-607 / 273	-77.3	-77.3	0.04 (5)	6.25	3- 6	-606 / 361	0.11 (1)
9- 3	-593 / 303	-77.3	-77.3	0.09 (5)	6.25	8- 9	-74 / 84	0.00 (1)
3- 4	-43 / 23	-77.3	-77.3	0.06 (1)	6.25			
6- 4	-91 / 67	0.0	0.0	0.06 (5)	7.81			
2- 8	-300 / 568	-17.5	-17.5	0.14 (1)	6.25			
8- 7	-300 / 568	-17.5	-17.5	0.16 (1)	6.25			
7- 6	-300 / 568	-17.5	-17.5	0.38 (1)	6.25			
6- 5	0 / 0	-17.5	-17.5	0.29 (1)	10.00			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	23.3	PSF
		DL =	3.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.0	PSF
TOTAL	LOAD	=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.10 (1-2:1) , BC=0.38 (6-7:1) , WB=0.11 (3-6:1) , SSI=0.19 (5-6:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (6) (INPUT = 0.90)
JSI METAL= 0.21 (6) (INPUT = 1.00)

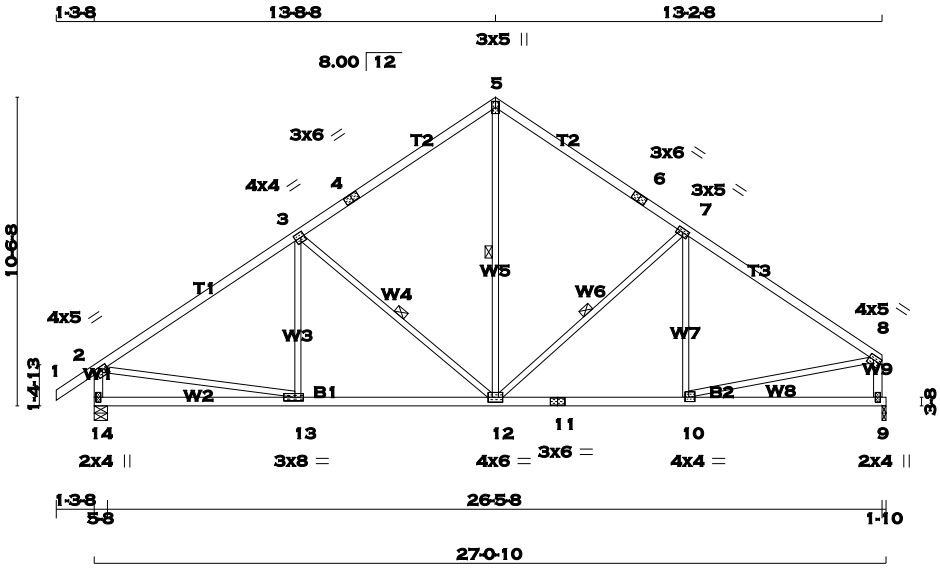
LICENSED PROFESSIONAL ENGINEER
T.L. WISE
100083566
PROVINCE OF ONTARIO

March 10, 2017

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

RECEIVED
TOWN OF MILTON
MAR 29, 2017
17-4978
BUILDING DIVISION



SCALE = 1:78.7

TOTAL WEIGHT = 2 X 114 = 228 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 5	2x4	DRY	No.2	SPF
5 - 6	2x4	DRY	No.2	SPF
6 - 8	2x4	DRY	No.2	SPF
14 - 2	2x4	DRY	No.2	SPF
9 - 8	2x4	DRY	No.2	SPF
14 - 11	2x4	DRY	No.2	SPF
11 - 9	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-t	MT20	4.0	5.0	1.50	2.00
3	TMWW-t	MT20	4.0	4.0	2.00	1.50
4	TS-t	MT20	3.0	6.0		
5	TTW+p	MT20	3.0	5.0		
6	TS-t	MT20	3.0	6.0		
7	TMWW-t	MT20	3.0	5.0		
8	TMVW-t	MT20	4.0	5.0	1.75	Edge
9	BMV1+p	MT20	2.0	4.0		
10	BMWW-t	MT20	4.0	4.0	1.75	1.50
11	BS-t	MT20	3.0	6.0		
12	BMWWW-t	MT20	4.0	6.0		
13	BMWW-t	MT20	3.0	8.0	1.50	3.50
14	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
14	1383	0	1470	452	-601	5-8	5-8
9	1276	0	1356	0	-533	1-10	1-10

PROVIDE ANCHORAGE AT BEARING JOINT 14 FOR 601 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 9 FOR 533 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 452 LBS FACTORED HORIZONTAL REACTION AT JOINT 14

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
14	968	691 / 0	0 / 0	0 / 0	219 / -607	277 / 0	0 / 0
9	896	627 / 0	0 / 0	0 / 0	199 / -554	269 / 0	0 / 0
HORIZONTAL REACTIONS							
14	---	0 / 0	0 / 0	0 / 0	323 / -309	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 14, 9

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 3-12, 5-12, 7-12. DBS = 20-0-0 . CBF = 68 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 29	-77.3	-77.3	0.10 (1)		10.00	13- 3	-69 / 160	0.04 (4)
2- 3	-1545 / 637	-77.3	-77.3	0.73 (7)		4.66	3-12	-626 / 554	0.33 (3)
3- 4	-1135 / 638	-77.3	-77.3	0.72 (7)		5.32	12- 5	-442 / 800	0.33 (7)
4- 5	-1135 / 638	-77.3	-77.3	0.72 (7)		5.32	12- 7	-543 / 509	0.28 (4)
5- 6	-1135 / 646	-77.3	-77.3	0.67 (8)		5.40	10- 7	-132 / 178	0.08 (3)
6- 7	-1135 / 646	-77.3	-77.3	0.67 (8)		5.40	2-13	-385 / 1326	0.34 (8)
7- 8	-1458 / 600	-77.3	-77.3	0.68 (8)		4.84	10- 8	-394 / 1262	0.33 (7)
14- 2	-1420 / 634	0.0	0.0	0.14 (1)		6.89			
9- 8	-1308 / 566	0.0	0.0	0.13 (1)		7.11			
14-13	-431 / 422	-17.5	-17.5	0.20 (11)		6.25			
13-12	-605 / 1399	-17.5	-17.5	0.30 (1)		6.25			
12-11	-358 / 1235	-17.5	-17.5	0.29 (1)		6.25			
11-10	-358 / 1235	-17.5	-17.5	0.29 (1)		6.25			
10- 9	-1 / 2	-1	(	1)		6.25			

WIND LOAD APPLIED IS CALCULATED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM/TERTIARY} WIN G MAY BE LOCATED ON {OPE LEAST {1-0-0} FT IN-SX AWAY FRO

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.90")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.90")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.73 (2-3:7) , BC=0.30 (12-13:1) , WB=0.34 (2-13:8) , SSI=0.23 (2-3:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (8) (INPUT = 0.90)
JSI METAL= 0.53 (2) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
T.L. WISE
100083566
PROVINCE OF ONTARIO

March 10, 2017

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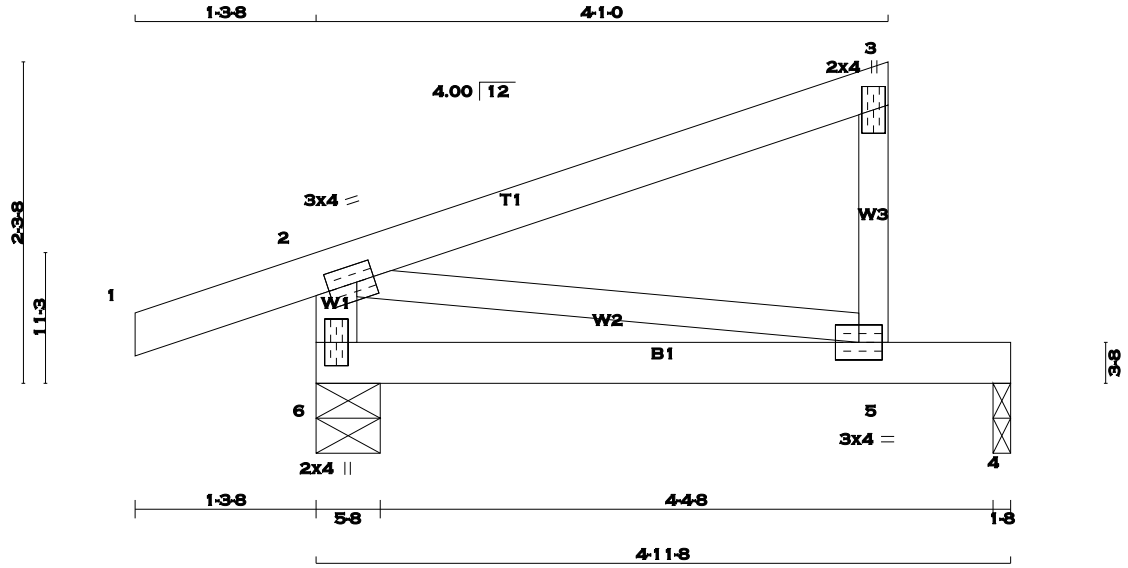

TOTAL WEIGHT = 5 X 17 = 85 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 3	2x4	DRY	No.2	SPF
5 - 3	2x3	DRY	No.2	SPF
6 - 2	2x4	DRY	No.2	SPF
6 - 4	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-t	MT20	3.0	4.0	1.50	1.50
3	TMV+p	MT20	2.0	4.0		
5	BMVW-t	MT20	3.0	4.0		
6	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
6	333	0	333	155	-174	5-8	5-8
4	173	0	173	0	-87	1-8	1-8

PROVIDE ANCHORAGE AT BEARING JOINT 6 FOR 174 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 4 FOR 150 LBS FACTORED UPLIFT

PROVIDE FOR 155 LBS FACTORED HORIZONTAL REACTION AT JOINT 6

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
6	231	174 / 0	0 / 0	0 / 0	0 / -161	57 / 0	0 / 0
4	123	78 / 0	0 / 0	0 / 0	0 / -91	45 / 0	0 / 0
HORIZONTAL REACTIONS							
6	---	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 6, 4

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 16	-77.3	-77.3	0.10 (1)	10.00	2- 5	0 / 120	0.02 (6)
2- 3	-59 / 11	-77.3	-77.3	0.22 (1)	6.25			
5- 3	-158 / 130	0.0	0.0	0.08 (5)	7.81			
6- 2	-261 / 181	0.0	0.0	0.03 (7)	7.81			
6- 5	-141 / 0	-17.5	-17.5	0.20 (1)	6.25			
5- 4	0 / 0	-17.5	-17.5	0.20 (1)	10.00			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	23.3	PSF
		DL =	3.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.0	PSF
TOTAL	LOAD	=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 872 (0.07")

CSI: TC=0.22 (2-3:1) , BC=0.20 (5-6:1) , WB=0.02 (2-5:6) , SSI=0.14 (4-5:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.26 (2) (INPUT = 0.90)
JSI METAL= 0.07 (6) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
T.L. WISE
100083566
PROVINCE OF ONTARIO
March 10, 2017

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

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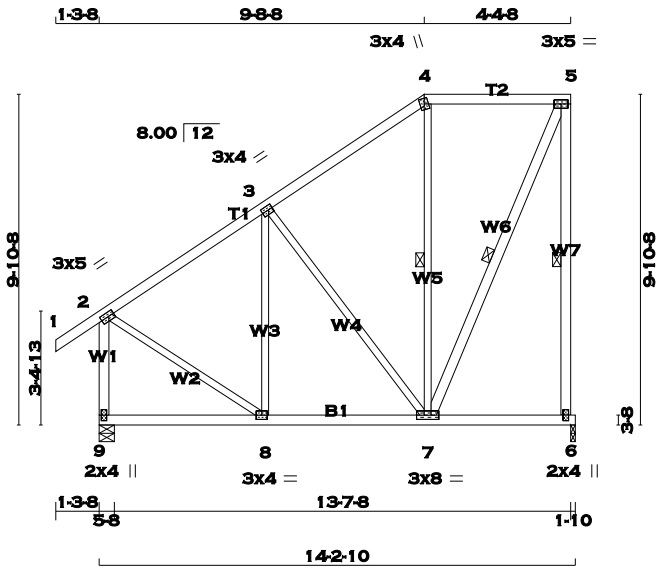
MAR 29, 2017
17-4978
BUILDING DIVISION



SCALE = 1:68.8

TOTAL WEIGHT = 2 X 85 = 170 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 5	2x4	DRY	No.2	SPF
6 - 5	2x4	DRY	No.2	SPF
9 - 2	2x4	DRY	No.2	SPF
9 - 6	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
7 - 5	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-t	MT20	3.0	5.0	1.50	2.00
3	TMWW-t	MT20	3.0	4.0	1.50	1.50
4	TTW+m	MT20	3.0	4.0		
5	TMVW-t	MT20	3.0	5.0		
6	BMV1+p	MT20	2.0	4.0		
7	BMWWW-t	MT20	3.0	8.0		
8	BMWW-t	MT20	3.0	4.0		
9	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

March 10, 2017

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
6	668	0	752	0	-447	1-10	1-10
9	774	0	814	555	-322	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 6 FOR 447 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 9 FOR 322 LBS FACTORED UPLIFT

PROVIDE FOR 555 LBS FACTORED HORIZONTAL REACTION AT JOINT 9

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
6	469	328 / 0	0 / 0	0 / 0	211 / -410	141 / 0	0 / 0
9	541	392 / 0	0 / 0	0 / 0	100 / -326	149 / 0	0 / 0
HORIZONTAL REACTIONS							
9	---	0 / 0	0 / 0	0 / 0	396 / -287	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 6, 9

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 5-6, 4-7, 5-7. DBS = 20-0-0 . CBF = 80 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 29	-77.3	-77.3	0.10 (1)	10.00	8- 3	-164 / 109	0.12 (4)
2- 3	-470 / 267	-77.3	-77.3	0.36 (7)	6.25	3- 7	-357 / 398	0.47 (3)
3- 4	-338 / 305	-77.3	-77.3	0.36 (7)	6.25	7- 4	-176 / 139	0.11 (3)
4- 5	-256 / 335	-77.3	-77.3	0.22 (7)	6.25	7- 5	-363 / 596	0.20 (7)
6- 5	-722 / 467	0.0	0.0	0.60 (7)	6.25	2- 8	-75 / 494	0.10 (1)
9- 2	-779 / 346	0.0	0.0	0.14 (1)	7.81			
9- 8	-494 / 374	-17.5	-17.5	0.10 (11)	6.25			
8- 7	-375 / 457	-17.5	-17.5	0.13 (11)	6.25			
7- 6	-73 / 188	-17.5	-17.5	0.08 (11)	6.25			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM A

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.47")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.47")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.60 (5-6:7) , BC=0.13 (7-8:11) , WB=0.47 (3-7:3) , SSI=0.16 (2-3:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

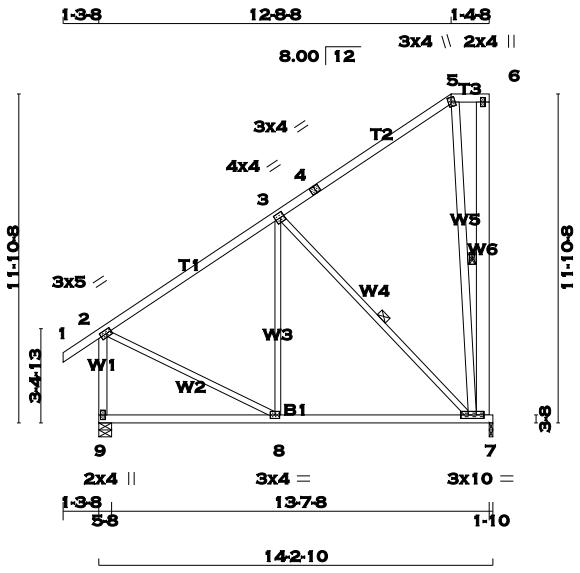
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (4) (INPUT = 0.90)
JSI METAL= 0.19 (2) (INPUT = 1.00)



SCALE = 1:83.1

TOTAL WEIGHT = 2 X 93 = 186 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 5	2x4	DRY	No.2	SPF
5 - 6	2x4	DRY	No.2	SPF
7 - 6	2x6	DRY	No.2	SPF
9 - 2	2x4	DRY	No.2	SPF
9 - 7	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
5 - 7	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-t	MT20	3.0	5.0	1.50	2.00
3	TMWW-t	MT20	4.0	4.0	2.00	1.50
4	TS-t	MT20	3.0	4.0		
5	TTW+m	MT20	3.0	4.0	2.00	1.25
6	TMV+p	MT20	2.0	4.0		
7	BMVWW1-t	MT20	3.0	10.0		
8	BMWW-t	MT20	3.0	4.0		
9	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
7	668	0	782	0	-490	1-10	1-10
9	774	0	836	667	-279	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 7 FOR 490 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 9 FOR 279 LBS FACTORED UPLIFT

PROVIDE FOR 667 LBS FACTORED HORIZONTAL REACTION AT JOINT 9

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
7	469	328 / 0	0 / 0	0 / 0	286 / -441	141 / 0	0 / 0
9	541	392 / 0	0 / 0	0 / 0	156 / -295	149 / 0	0 / 0
HORIZONTAL REACTIONS							
9	---	0 / 0	0 / 0	0 / 0	477 / -336	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 7, 9

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 6-7, 3-7, 5-7. DBS = 20-0-0 . CBF = 66 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
1- 2	0 / 29	-77.3	-77.3	0.10 (1)	10.00
2- 3	-501 / 206	-77.3	-77.3	0.58 (7)	6.25
3- 4	-165 / 230	-77.3	-77.3	0.57 (7)	6.25
4- 5	-165 / 230	-77.3	-77.3	0.57 (7)	6.25
5- 6	-89 / 227	-77.3	-77.3	0.05 (7)	6.25
7- 6	-56 / 41	0.0	0.0	0.39 (7)	6.25
9- 2	-796 / 307	0.0	0.0	0.14 (4)	7.81
9- 8	-607 / 442	-17.5	-17.5	0.26 (11)	6.25
8- 7	-373 / 464	-17.5	-17.5	0.28 (11)	6.25

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
8- 3	-62 / 163	0.07 (4)
3- 7	-611 / 553	0.45 (3)
2- 8	-120 / 501	0.11 (5)
5- 7	-250 / 196	0.19 (3)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM [illegible]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.47")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.47")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.58 (2-3:7) , BC=0.28 (7-8:11) , WB=0.45 (3-7:3) , SSI=0.21 (2-3:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.67 (2) (INPUT = 0.90)
JSI METAL= 0.21 (2) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
T.L. WISE
100083566
PROVINCE OF ONTARIO

March 10, 2017

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

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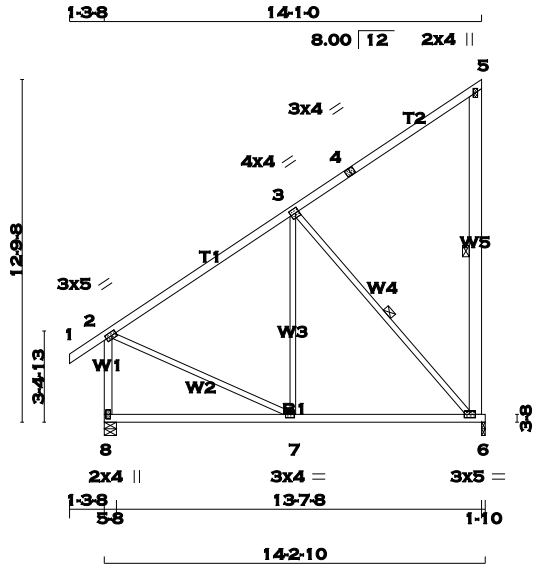

SCALE = 1:86.0

TOTAL WEIGHT = 2 X 83 = 166 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 5	2x4	DRY	No.2	SPF
6 - 5	2x6	DRY	No.2	SPF
8 - 2	2x4	DRY	No.2	SPF
8 - 6	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-t	MT20	3.0	5.0	1.50	2.00
3	TMWW-t	MT20	4.0	4.0	2.00	1.50
4	TS-t	MT20	3.0	4.0		
5	TMV+p	MT20	2.0	4.0		
6	BMVW1-t	MT20	3.0	5.0	1.50	2.25
7	BMWW-t	MT20	3.0	4.0		
8	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
6	668	0	802	0	-512	1-10	1-10
8	774	0	855	766	-257	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 6 FOR 512 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 8 FOR 257 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 766 LBS FACTORED HORIZONTAL REACTION AT JOINT 8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
6	469	328 / 0	0 / 0	0 / 0	336 / -456	141 / 0	0 / 0
8	541	392 / 0	0 / 0	0 / 0	202 / -279	149 / 0	0 / 0
HORIZONTAL REACTIONS							
8	---	0 / 0	0 / 0	0 / 0	547 / -381	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 6, 8

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 5-6, 3-6. DBS = 20-0-0 . CBF = 75 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 29	-77.3	-77.3	0.10 (1)	10.00	7- 3	-51 / 177	0.06 (4)
2- 3	-513 / 176	-77.3	-77.3	0.70 (7)	6.25	3- 6	-689 / 624	0.52 (3)
3- 4	-228 / 286	-77.3	-77.3	0.70 (7)	6.25	2- 7	-166 / 509	0.19 (5)
4- 5	-228 / 286	-77.3	-77.3	0.70 (7)	6.25			
6- 5	-245 / 222	0.0	0.0	0.45 (7)	6.25			
8- 2	-808 / 290	0.0	0.0	0.15 (4)	7.81			
8- 7	-706 / 505	-17.5	-17.5	0.25 (11)	6.25			
7- 6	-400 / 462	-17.5	-17.5	0.28 (11)	6.25			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPE[illegible]} TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FRO[illegible]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.47")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.47")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.70 (2-3:7) , BC=0.28 (6-7:11) , WB=0.52 (3-6:3) , SSI=0.23 (2-3:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (4) (INPUT = 0.90)
JSI METAL= 0.24 (4) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
T.L. WISE
100083566
PROVINCE OF ONTARIO

March 10, 2017

KOTT

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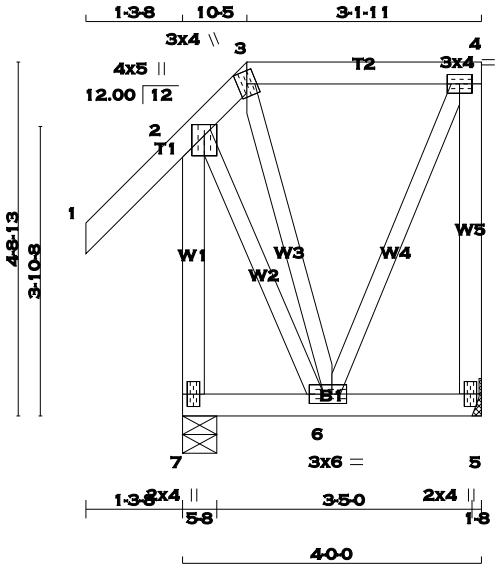

SCALE = 1:30.8

TOTAL WEIGHT = 2 X 31 = 62 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 3	2x4	DRY	No.2	SPF
3 - 4	2x4	DRY	No.2	SPF
5 - 4	2x4	DRY	No.2	SPF
7 - 2	2x4	DRY	No.2	SPF
7 - 5	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW+p	MT20	4.0	5.0	1.75	2.00
3	TTW+m	MT20	3.0	4.0		
4	TMVW-t	MT20	3.0	4.0		
5	BMV1+p	MT20	2.0	4.0		
6	BMWWW-t	MT20	3.0	6.0		
7	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
5	171	0	215	0	-208	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
7	316	0	348	277	-99	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 5 FOR 208 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 7 FOR 150 LBS FACTORED UPLIFT

PROVIDE FOR 277 LBS FACTORED HORIZONTAL REACTION AT JOINT 7

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
5	120	82 / 0	0 / 0	0 / 0	111 / -174	39 / 0	0 / 0
7	219	169 / 0	0 / 0	0 / 0	78 / -103	50 / 0	0 / 0
HORIZONTAL REACTIONS							
7	---	0 / 0	0 / 0	0 / 0	198 / -173	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 7

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 38	-77.3	-77.3	0.11 (7)	10.00	3- 6	-67 / 40	0.02 (7)
2- 3	-123 / 191	-77.3	-77.3	0.14 (7)	6.25	6- 4	-173 / 146	0.06 (6)
3- 4	-35 / 123	-77.3	-77.3	0.13 (1)	6.25	2- 6	-159 / 215	0.04 (5)
5- 4	-198 / 217	0.0	0.0	0.14 (7)	7.81			
7- 2	-333 / 106	0.0	0.0	0.09 (7)	7.81			
7- 6	-208 / 210	-17.5	-17.5	0.04 (5)	6.25			
6- 5	-34 / 87	-17.5	-17.5	0.02 (7)	6.25			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.14 (2-3:7) , BC=0.04 (6-7:5) , WB=0.06 (4-6:6) , SSI=0.09 (3-4:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.48 (3) (INPUT = 0.90)
JSI METAL= 0.09 (3) (INPUT = 1.00)

March 10, 2017

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

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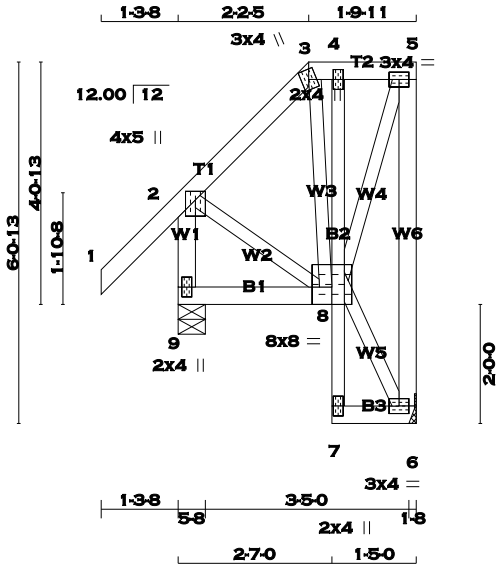

SCALE = 1:38.7

TOTAL WEIGHT = 2 X 35 = 69 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 3	2x4	DRY	No.2	SPF
3 - 5	2x4	DRY	No.2	SPF
6 - 5	2x4	DRY	No.2	SPF
9 - 2	2x4	DRY	No.2	SPF
9 - 8	2x4	DRY	No.2	SPF
7 - 4	2x3	DRY	No.2	SPF
7 - 6	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW+p	MT20	4.0	5.0	1.75	2.00
3	TTW+m	MT20	3.0	4.0		
4	TMV+p	MT20	2.0	4.0		
5	TMVW-t	MT20	3.0	4.0		
6	BMVW1-t	MT20	3.0	4.0		
7	BMV+p	MT20	2.0	4.0		
8	BVMWWWW*-t	MT20	8.0	8.0	Edge	1.50
9	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
6	190	0	248	274	-295	IN-SX	IN-SX
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	
9	298	0	360	0	-133	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 6 FOR 295 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 9 FOR 150 LBS FACTORED UPLIFT

PROVIDE FOR 274 LBS FACTORED HORIZONTAL REACTION AT JOINT 6

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
6	133	93 / 0	0 / 0	0 / 0	145 / -236	40 / 0	0 / 0
9	206	158 / 0	0 / 0	0 / 0	157 / -126	48 / 0	0 / 0
HORIZONTAL REACTIONS							
6	---	0 / 0	0 / 0	0 / 0	252 / -179	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 9

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	VERT. LOAD FROM	LC1 TO	MAX (PLF) CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 38	-77.3	-77.3	0.11 (7)	10.00	3- 8	-81 / 120	0.02 (6)
2- 3	-138 / 146	-77.3	-77.3	0.10 (7)	6.25	8- 6	-263 / 421	0.08 (6)
3- 4	-92 / 118	-77.3	-77.3	0.03 (5)	6.25	8- 5	-32 / 141	0.03 (1)
4- 5	-90 / 116	-77.3	-77.3	0.03 (8)	6.25	2- 8	-151 / 241	0.05 (6)
6- 5	-185 / 49	0.0	0.0	0.19 (7)	7.81			
9- 2	-337 / 150	0.0	0.0	0.03 (4)	7.81			
9- 8	-14 / 30	-17.5	-17.5	0.04 (11)	6.25			
7- 8	0 / 13	0.0	0.0	0.01 (4)	10.00			
8- 4	-80 / 75	0.0	0.0	0.02 (6)	7.81			
7- 6	0 / 1	-17.5	-17.5	0.01 (11)	10.00			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.19 (5-6:7) , BC=0.04 (8-9:11) , WB=0.08 (6-8:6) , SSI=0.08 (5-6:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (6) (INPUT = 0.90)
JSI METAL= 0.12 (6) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
T.L. WISE
100083566
PROVINCE OF ONTARIO
March 10, 2017

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

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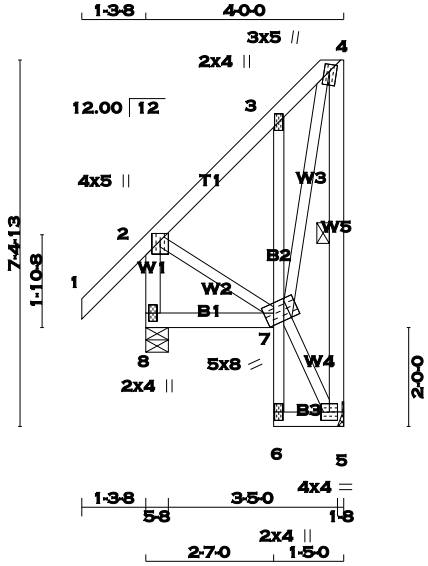

SCALE = 1:46.5

TOTAL WEIGHT = 2 X 36 = 72 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
5 - 4	2x4	DRY	No.2	SPF
8 - 2	2x4	DRY	No.2	SPF
8 - 7	2x4	DRY	No.2	SPF
6 - 3	2x3	DRY	No.2	SPF
6 - 5	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW+p	MT20	4.0	5.0	1.75	2.00
3	TMV+p	MT20	2.0	4.0		
4	TMVW+w	MT20	3.0	5.0	1.75	1.25
5	BMVW1-t	MT20	4.0	4.0		
6	BMV+p	MT20	2.0	4.0		
7	BVMWWW-w	MT20	5.0	8.0	2.75	2.25
8	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
5	190	0	329	401	-482	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
8	298	0	415	0	-242	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 5 FOR 482 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 8 FOR 242 LBS FACTORED UPLIFT

PROVIDE FOR 401 LBS FACTORED HORIZONTAL REACTION AT JOINT 5

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
5	133	93 / 0	0 / 0	0 / 0	251 / -370	40 / 0	0 / 0
8	206	158 / 0	0 / 0	0 / 0	294 / -204	48 / 0	0 / 0
HORIZONTAL REACTIONS							
5	---	0 / 0	0 / 0	0 / 0	343 / -236	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 8

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 4-5. DBS = 20-0-0 . CBF = 22 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 38	-77.3	-77.3	0.11 (7)	10.00	7- 5	-332 / 603	0.12 (6)
2- 3	-311 / 245	-77.3	-77.3	0.09 (7)	6.25	7- 4	-134 / 257	0.06 (5)
3- 4	-189 / 247	-77.3	-77.3	0.11 (7)	6.25	2- 7	-231 / 379	0.07 (6)
5- 4	-175 / 0	0.0	0.0	0.33 (8)	6.25			
8- 2	-392 / 259	0.0	0.0	0.04 (5)	7.81			
8- 7	-14 / 30	-17.5	-17.5	0.04 (11)	6.25			
6- 7	0 / 13	0.0	0.0	0.01 (4)	10.00			
7- 3	-245 / 333	0.0	0.0	0.07 (7)	7.81			
6- 5	-1 / 2	-17.5	-17.5	0.01 (11)	10.00			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FRO[illegible]VE

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.33 (4-5:8) , BC=0.07 (3-7:7) , WB=0.12 (5-7:6) , SSI=0.10 (4-5:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.64 (5) (INPUT = 0.90)
JSI METAL= 0.16 (5) (INPUT = 1.00)

March 10, 2017

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

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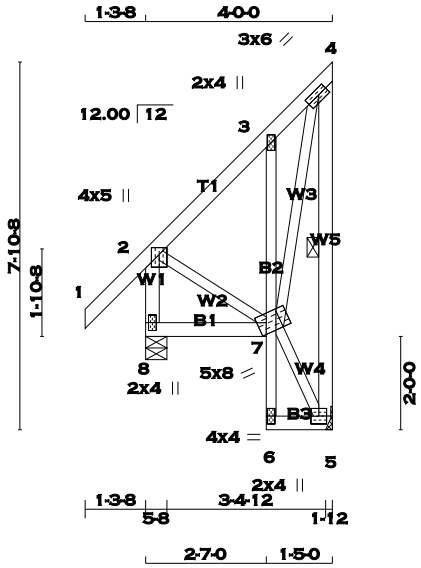

SCALE = 1:49.3

TOTAL WEIGHT = 4 X 36 = 146 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
8 - 2	2x4	DRY	No.2	SPF
1 - 4	2x4	DRY	No.2	SPF
5 - 4	2x4	DRY	No.2	SPF
8 - 7	2x4	DRY	No.2	SPF
6 - 3	2x3	DRY	No.2	SPF
6 - 5	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW+p	MT20	4.0	5.0	1.75	2.00
3	TMV+p	MT20	2.0	4.0		
4	TMVW-t	MT20	3.0	6.0	1.50	2.50
5	BMVW1-t	MT20	4.0	4.0		
6	BMV+p	MT20	2.0	4.0		
7	BVMWWW-w	MT20	5.0	8.0	2.75	2.25
8	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
8	298	0	415	0	-242	5-8	5-8
5	190	0	329	401	-482	HANGER BY OTHERS MIN. SEAT SIZE: 1-12	

PROVIDE ANCHORAGE AT BEARING JOINT 8 FOR 242 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 5 FOR 482 LBS FACTORED UPLIFT

PROVIDE FOR 401 LBS FACTORED HORIZONTAL REACTION AT JOINT 5

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
8	206	158 / 0	0 / 0	0 / 0	294 / -204	48 / 0	0 / 0
5	133	93 / 0	0 / 0	0 / 0	251 / -370	40 / 0	0 / 0
HORIZONTAL REACTIONS							
5	---	0 / 0	0 / 0	0 / 0	343 / -236	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 8

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 4-5. DBS = 20-0-0 . CBF = 22 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
8- 2	-392 / 259	0.0	0.0	0.04 (5)	7.81
1- 2	0 / 38	-77.3	-77.3	0.11 (7)	10.00
2- 3	-311 / 245	-77.3	-77.3	0.09 (7)	6.25
3- 4	-189 / 247	-77.3	-77.3	0.11 (7)	6.25
5- 4	-175 / 0	0.0	0.0	0.33 (8)	6.25
8- 7	-14 / 30	-17.5	-17.5	0.04 (11)	6.25
6- 7	0 / 13	0.0	0.0	0.01 (4)	10.00
7- 3	-245 / 333	0.0	0.0	0.07 (7)	7.81
6- 5	-1 / 2	-17.5	-17.5	0.01 (11)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
2- 7	-231 / 379	0.07 (6)
7- 5	-332 / 603	0.12 (6)
7- 4	-134 / 257	0.06 (5)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM [illegible]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.33 (4-5:8) , BC=0.07 (3-7:7) , WB=0.12 (5-7:6) , SSI=0.10 (4-5:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.64 (5) (INPUT = 0.90)
JSI METAL= 0.16 (5) (INPUT = 1.00)

March 10, 2017

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

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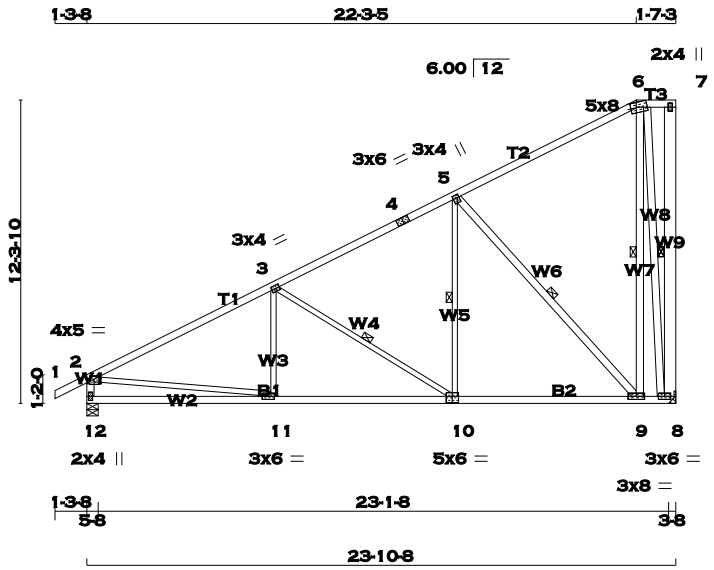


SCALE = 1:93.4

TOTAL WEIGHT = 2 X 143 = 286 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 6	2x4	DRY	No.2	SPF
6 - 7	2x4	DRY	No.2	SPF
8 - 7	2x6	DRY	No.2	SPF
12 - 2	2x4	DRY	No.2	SPF
12 - 10	2x4	DRY	No.2	SPF
10 - 8	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
5 - 9	2x4	DRY	No.2	SPF
9 - 6	2x4	DRY	No.2	SPF
6 - 8	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-p	MT20	4.0	5.0	1.50	2.25
3	TMWW-t	MT20	3.0	4.0	1.50	1.75
4	TS-t	MT20	3.0	6.0		
5	TMWW+t	MT20	3.0	4.0	1.50	0.75
6	TTWW-m	MT20	5.0	8.0	2.25	2.75
7	TMV+p	MT20	2.0	4.0		
8	BMVW1-t	MT20	3.0	6.0		
9	BMWW-t	MT20	3.0	8.0		
10	BSWW-l	MT20	5.0	6.0	3.25	3.00
11	BMWW-t	MT20	3.0	6.0	1.50	1.75
12	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
8	1132	0	1191	0	-685	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	
12	1237	0	1272	725	-573	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 8 FOR 685 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 12 FOR 573 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 725 LBS FACTORED HORIZONTAL REACTION AT JOINT 12

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
8	795	556 / 0	0 / 0	0 / 0	146 / -643	239 / 0	0 / 0
12	866	619 / 0	0 / 0	0 / 0	86 / -568	247 / 0	0 / 0
HORIZONTAL REACTIONS							
12	---	0 / 0	0 / 0	0 / 0	518 / -122	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 12

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.61 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 7-8, 3-10, 5-10, 6-9. DBS = 20-0-0 . CBF = 81 LBS.
1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 5-9. DBS = 14-0-0 . CBF = 90 LBS.
1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 6-8. DBS = 12-0-0 . CBF = 82 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 23	-77.3	-77.3	0.10 (1)	10.00	11- 3	-21 / 160	0.04 (11)
2- 3	-1574 / 682	-77.3	-77.3	0.62 (7)	4.61	3-10	-688 / 517	0.35 (3)
3- 4	-1002 / 504	-77.3	-77.3	0.59 (7)	5.76	10- 5	-189 / 505	0.11 (1)
4- 5	-1002 / 504	-77.3	-77.3	0.59 (7)	5.76	5- 9	-1085 / 771	0.73 (3)
5- 6	-270 / 231	-77.3	-77.3	0.54 (7)	6.25	9- 6	-464 / 898	0.39 (7)
6- 7	-92 / 235	-77.3	-77.3	0.06 (7)	6.25	6- 8	-1144 / 601	0.96 (3)
8- 7	-65 / 49	0.0	0.0	0.42 (7)	6.25	2-11	-455 / 1448	0.51 (7)
12- 2	-1217 / 610	0.0	0.0	0.12 (1)	7.30			
12-11	-707 / 162	-17.5	-17.5	0.23 (11)	6.25			
11-10	-792 / 1445	-17.5	-17.5	0.34 (1)	6.25			
10- 9	-44 / 8	-17	-17	0	6.25			
9- 8	/ 2	-17	-17	0.1 (11)	6.25			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF {0.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COE ... STING SYSTEM}.INTERNAL WIN ... G MAY BE ... PSF {OPE ... LEAST {0-0} FT-IN-SX AWAY FRO ...

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.80")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.80")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.62 (2-3:7) , BC=0.34 (10-11:1) , WB=0.96 (6-8:3) , SSI=0.24 (2-3:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

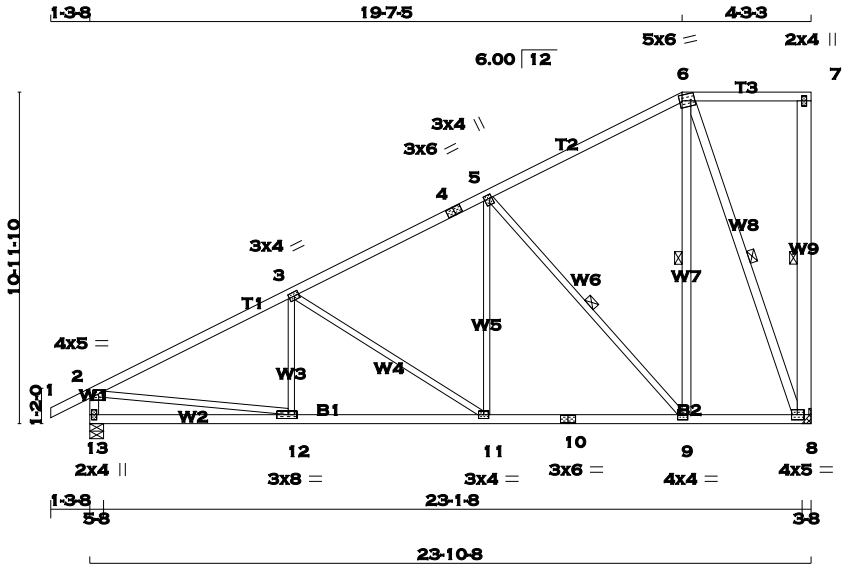
JSI GRIP= 0.90 (11) (INPUT = 0.90)
JSI METAL= 0.41 (2) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
T.L. WISE
100083566
PROVINCE OF ONTARIO

March 10, 2017

KOTT

RECEIVED
TOWN OF MILTON
MAR 29 2017
17-4978
BUILDING DIVISION



SCALE = 1:76.3

TOTAL WEIGHT = 2 X 131 = 261 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 6	2x4	DRY	No.2	SPF
6 - 7	2x4	DRY	No.2	SPF
8 - 7	2x6	DRY	No.2	SPF
13 - 2	2x4	DRY	No.2	SPF
13 - 10	2x4	DRY	No.2	SPF
10 - 8	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
9 - 6	2x4	DRY	No.2	SPF
6 - 8	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-p	MT20	4.0	5.0	1.50	2.25
3	TMWW-t	MT20	3.0	4.0	1.50	1.75
4	TS-t	MT20	3.0	6.0		
5	TMWW+t	MT20	3.0	4.0	1.50	0.75
6	TTWW-m	MT20	5.0	6.0	2.25	1.00
7	TMV+p	MT20	2.0	4.0		
8	BMVW1-t	MT20	4.0	5.0	2.00	2.25
9	BMWW-t	MT20	4.0	4.0	2.00	1.75
10	BS-t	MT20	3.0	6.0		
11	BMWW-t	MT20	3.0	4.0		
12	BMWW-t	MT20	3.0	8.0	1.50	3.50
13	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
8	1132	0	1188	0	-667	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	
13	1237	0	1274	647	-591	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 8 FOR 667 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 13 FOR 591 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 647 LBS FACTORED HORIZONTAL REACTION AT JOINT 13

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
8	795	556 / 0	0 / 0	0 / 0	140 / -630	239 / 0	0 / 0
13	866	619 / 0	0 / 0	0 / 0	91 / -581	247 / 0	0 / 0
HORIZONTAL REACTIONS							
13	---	0 / 0	0 / 0	0 / 0	462 / -111	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 13

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.80 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 7-8, 6-9. DBS = 20-0-0 . CBF = 48 LBS.
1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 5-9. DBS = 16-0-0 . CBF = 88 LBS.
1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 6-8. DBS = 14-0-0 . CBF = 89 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 23	-77.3	-77.3	0.10 (1)	10.00	12- 3	-59 / 161	0.03 (11)
2- 3	-1587 / 730	-77.3	-77.3	0.51 (7)	4.80	3-11	-556 / 431	0.66 (3)
3- 4	-1122 / 599	-77.3	-77.3	0.49 (7)	5.65	11- 5	-155 / 418	0.17 (7)
4- 5	-1122 / 599	-77.3	-77.3	0.49 (7)	5.65	5- 9	-929 / 666	0.64 (3)
5- 6	-502 / 372	-77.3	-77.3	0.44 (7)	6.25	9- 6	-420 / 798	0.25 (7)
6- 7	-82 / 209	-77.3	-77.3	0.24 (1)	6.25	6- 8	-1067 / 591	0.74 (3)
8- 7	-174 / 133	0.0	0.0	0.34 (7)	6.25	2-12	-512 / 1451	0.40 (7)
13- 2	-1225 / 624	0.0	0.0	0.12 (1)	7.29			
13-12	-629 / 147	-17.5	-17.5	0.18 (11)	6.25			
12-11	-809 / 1465	-17.5	-17.5	0.31 (1)	6.25			
11-10	-490 / 999	-17.5	-17.5	0.23 (1)	6.25			
10- 9	-49 / 99	-17	-17	0.1 (11)	6.25			
9- 8	/ 39	-17	-17	0.1 (11)	6.25			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF {0.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COE STING SYSTEM}.INTERNAL WIN C MAY BE ASSIGNED {OPE LEAST {0-0} FT-IN-SX AWAY FRO

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.80")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.80")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.51 (2-3:7) , BC=0.31 (11-12:1) , WB=0.74 (6-8:3) , SSI=0.21 (2-3:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (12) (INPUT = 0.90)
JSI METAL= 0.41 (2) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
T.L. WISE
100083566
PROVINCE OF ONTARIO
March 10, 2017

KOTT

RECEIVED
TOWN OF MILTON
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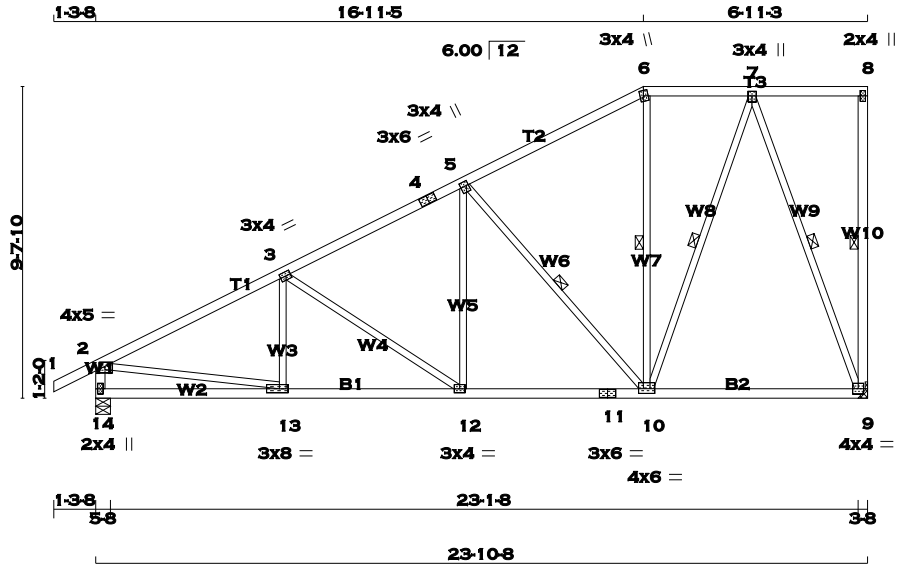
17-4978
BUILDING DIVISION



SCALE = 1:71.3

TOTAL WEIGHT = 2 X 117 = 234 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 6	2x4	DRY	No.2	SPF
6 - 8	2x4	DRY	No.2	SPF
9 - 8	2x4	DRY	No.2	SPF
14 - 2	2x4	DRY	No.2	SPF
14 - 11	2x4	DRY	No.2	SPF
11 - 9	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-p	MT20	4.0	5.0	1.50	2.25
3	TMWW-t	MT20	3.0	4.0	1.50	1.75
4	TS-t	MT20	3.0	6.0		
5	TMWW+t	MT20	3.0	4.0	1.75	0.75
6	TTW+m	MT20	3.0	4.0	2.00	1.25
7	TMWW+t	MT20	3.0	4.0	1.75	1.50
8	TMV+p	MT20	2.0	4.0		
9	BMVW1-t	MT20	4.0	4.0		
10	BMWWW-t	MT20	4.0	6.0	1.75	3.00
11	BS-t	MT20	3.0	6.0		
12	BMWW-t	MT20	3.0	4.0		
13	BMWW-t	MT20	3.0	8.0	1.50	3.25
14	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
9	1132	0	1187	0	-651	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	
14	1237	0	1276	568	-607	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 9 FOR 651 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 14 FOR 607 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 568 LBS FACTORED HORIZONTAL REACTION AT JOINT 14

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
9	795	556 / 0	0 / 0	0 / 0	136 / -618	239 / 0	0 / 0
14	866	619 / 0	0 / 0	0 / 0	96 / -593	247 / 0	0 / 0
HORIZONTAL REACTIONS							
14	---	0 / 0	0 / 0	0 / 0	406 / -100	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 14

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.94 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 8-9, 5-10, 6-10, 7-10. DBS = 20-0-0 . CBF = 90 LBS.
1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 7-9. DBS = 14-0-0 . CBF = 92 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 23	-77.3	-77.3	0.10 (1)	10.00	13- 3	-104 / 167	0.03 (7)
2- 3	-1604 / 767	-77.3	-77.3	0.41 (7)	4.94	3-12	-420 / 340	0.34 (3)
3- 4	-1247 / 688	-77.3	-77.3	0.40 (7)	5.51	12- 5	-129 / 322	0.10 (7)
4- 5	-1247 / 688	-77.3	-77.3	0.40 (7)	5.51	5-10	-764 / 569	0.38 (3)
5- 6	-734 / 502	-77.3	-77.3	0.36 (7)	6.25	10- 6	-13 / 81	0.02 (11)
6- 7	-638 / 527	-77.3	-77.3	0.18 (7)	6.25	10- 7	-350 / 717	0.24 (7)
7- 8	-72 / 183	-77.3	-77.3	0.15 (1)	6.25	7- 9	-1098 / 641	0.77 (3)
9- 8	-112 / 86	0.0	0.0	0.52 (7)	6.25	2-13	-559 / 1457	0.32 (1)
14- 2	-1233 / 635	0.0	0.0	0.12 (1)	7.27			
14-13	-550 / 131	-17.5	-17.5	0.14 (11)	6.25			
13-12	-816 / 1474	-17.5	-17.5	0.29 (1)	6.25			
12-11	-53 / 1	-17	-17	0	6.25			
11-10	/ 1	-17	-17.5	0.2 1)	6.25			
10- 9	/ 3	-17	-17.5	0.2 11)	6.25			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF {40-0 C EXTERNAL PEAK
COE STING SYSTEM) INTERNAL
WIN G MAY BE LOCATED ON
{OPE LEAST {0-0} FT IN SX AWAY
FRO

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.80")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.80")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.52 (8-9:7) , BC=0.29 (12-13:1) , WB=0.77 (7-9:3) , SSI=0.18 (2-3:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

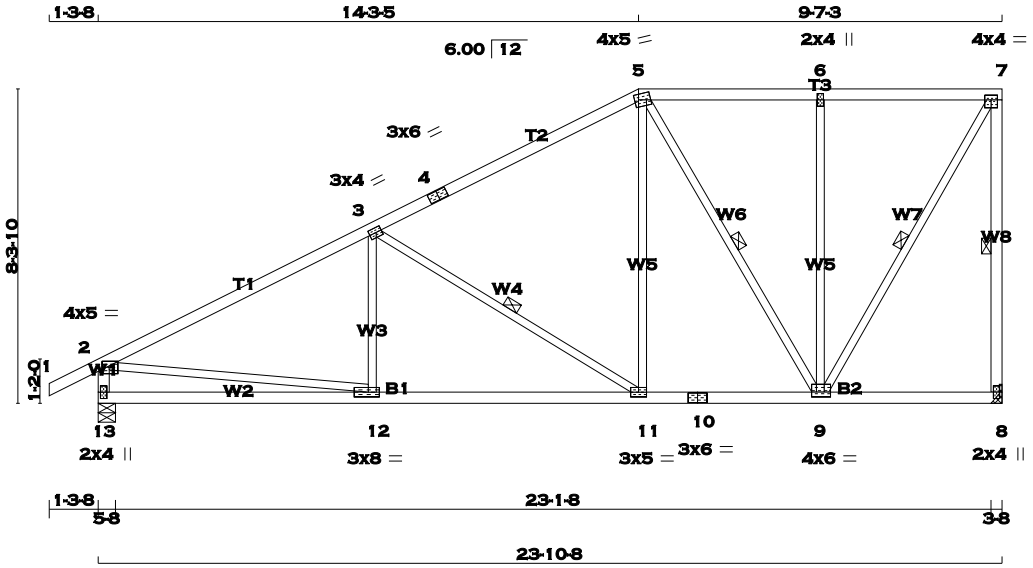
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (13) (INPUT = 0.90)
JSI METAL= 0.41 (2) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
T.L. WISE
100083566
PROVINCE OF ONTARIO
March 10, 2017

KOTT

RECEIVED
TOWN OF MILTON
MAR 17 2017
BUILDING DIVISION



SCALE = 1:60.9

TOTAL WEIGHT = 2 X 111 = 221 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 5	2x4	DRY	No.2	SPF
5 - 7	2x4	DRY	No.2	SPF
8 - 7	2x4	DRY	No.2	SPF
13 - 2	2x4	DRY	No.2	SPF
13 - 10	2x4	DRY	No.2	SPF
10 - 8	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-p	MT20	4.0	5.0	1.50	2.25
3	TMWW-t	MT20	3.0	4.0	1.50	1.75
4	TS-t	MT20	3.0	6.0		
5	TTWW-m	MT20	4.0	5.0	1.75	1.25
6	TMW+w	MT20	2.0	4.0		
7	TMVW-t	MT20	4.0	4.0	1.50	2.00
8	BMV1+p	MT20	2.0	4.0		
9	BMWWW-t	MT20	4.0	6.0	1.50	1.50
10	BS-t	MT20	3.0	6.0		
11	BMWW-t	MT20	3.0	5.0		
12	BMWW-t	MT20	3.0	8.0	1.50	3.50
13	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
8	1132	0	1185	0	-637	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	
13	1237	0	1277	490	-621	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 8 FOR 637 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 13 FOR 621 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 490 LBS FACTORED HORIZONTAL REACTION AT JOINT 13

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
8	795	556 / 0	0 / 0	0 / 0	132 / -608	239 / 0	0 / 0
13	866	619 / 0	0 / 0	0 / 0	100 / -603	247 / 0	0 / 0
HORIZONTAL REACTIONS							
13	---	0 / 0	0 / 0	0 / 0	350 / -89	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 13

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 7-8. DBS = 12-0-0 . CBF = 82 LBS.
1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 3-11, 5-9, 7-9. DBS = 20-0-0 . CBF = 87 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 23	-77.3	-77.3	0.10 (1)	10.00	12- 3	-29 / 175	0.04 (11)
2- 3	-1598 / 792	-77.3	-77.3	0.70 (7)	4.58	3-11	-736 / 560	0.34 (3)
3- 4	-981 / 596	-77.3	-77.3	0.67 (7)	5.61	11- 5	-229 / 494	0.31 (7)
4- 5	-981 / 596	-77.3	-77.3	0.67 (7)	5.61	5- 9	-504 / 280	0.29 (3)
5- 6	-611 / 484	-77.3	-77.3	0.27 (7)	6.25	9- 6	-483 / 372	0.65 (3)
6- 7	-611 / 484	-77.3	-77.3	0.27 (7)	6.25	9- 7	-636 / 1166	0.37 (7)
8- 7	-1149 / 660	0.0	0.0	0.46 (7)	5.97	2-12	-564 / 1455	0.56 (7)
13- 2	-1225 / 656	0.0	0.0	0.12 (1)	7.29			
13-12	-472 / 116	-17.5	-17.5	0.23 (11)	6.25			
12-11	-785 / 1473	-17.5	-17.5	0.34 (1)	6.25			
11-10	-376 / 856	-17.5	-17.5	0.20 (1)	6.25			
10- 9	-37 / 85	-17	-17	0.0 (11)	6.25			
9- 8	/ 1	-17	-17	0.0 (11)	6.25			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF {0.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COE STING SYSTEM}.INTERNAL WIN G MAY BE ADDED OR {OPE LEAST {0-0} FT-IN-SX AWAY FRO

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.80")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.80")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.70 (2-3:7) , BC=0.34 (11-12:1) , WB=0.65 (6-9:3) , SSI=0.24 (2-3:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (7) (INPUT = 0.90)
JSI METAL= 0.42 (2) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
T.L. WISE
100083566
PROVINCE OF ONTARIO

March 10, 2017

KOTT

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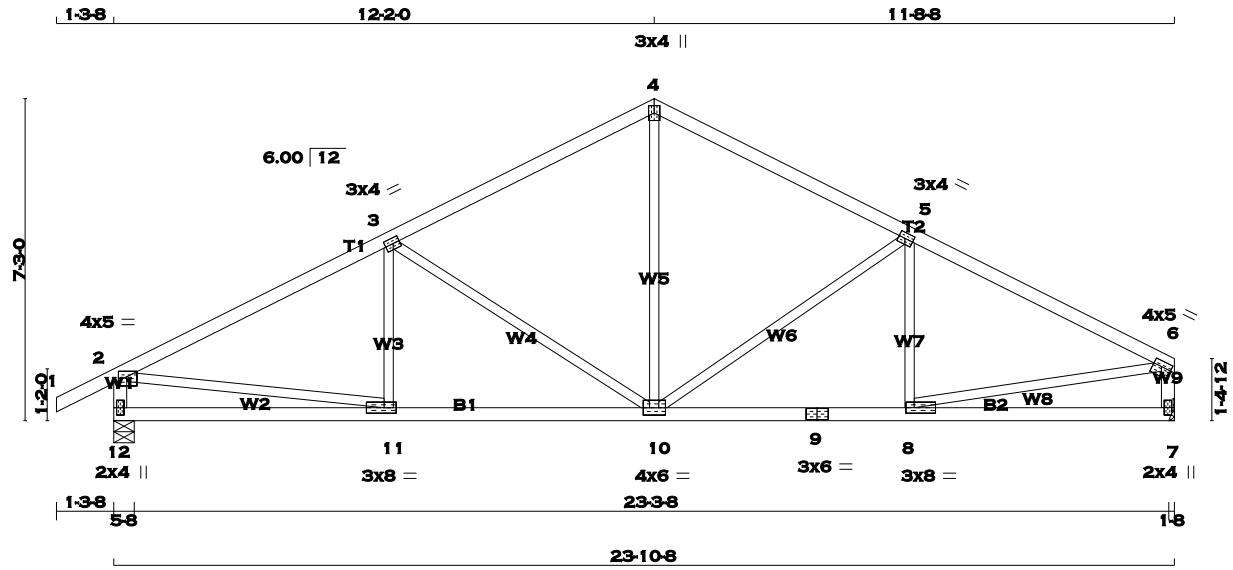

SCALE = 1:51.9

TOTAL WEIGHT = 2 X 93 = 187 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 6	2x4	DRY	No.2	SPF
12 - 2	2x4	DRY	No.2	SPF
7 - 6	2x4	DRY	No.2	SPF
12 - 9	2x4	DRY	No.2	SPF
9 - 7	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-p	MT20	4.0	5.0	1.50	2.25
3	TMWW-t	MT20	3.0	4.0	1.50	1.75
4	TTW+p	MT20	3.0	4.0		
5	TMWW-t	MT20	3.0	4.0	1.50	1.75
6	TMVW-t	MT20	4.0	5.0	1.50	2.25
7	BMV1+p	MT20	2.0	4.0		
8	BMWW-t	MT20	3.0	8.0	1.50	2.25
9	BS-t	MT20	3.0	6.0		
10	BMWWW-t	MT20	4.0	6.0		
11	BMWW-t	MT20	3.0	8.0	1.50	3.25
12	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
12	1237	0	1260	201	-558	5-8	5-8
7	1132	0	1157	0	-491	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

PROVIDE ANCHORAGE AT BEARING JOINT 12 FOR 558 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 7 FOR 491 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 201 LBS FACTORED HORIZONTAL REACTION AT JOINT 12

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
12	866	619 / 0	0 / 0	0 / 0	57 / -558	247 / 0	0 / 0
7	795	556 / 0	0 / 0	0 / 0	62 / -504	239 / 0	0 / 0
HORIZONTAL REACTIONS							
12	---	0 / 0	0 / 0	0 / 0	144 / -118	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 12

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.83 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FROM	VERT. LOAD (PLF) TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 23	-77.3	-77.3	0.10 (1)	10.00	11- 3	-83 / 159	0.03 (7)	
2- 3	-1587 / 693	-77.3	-77.3	0.52 (7)	4.83	3-10	-556 / 458	0.54 (3)	
3- 4	-1142 / 576	-77.3	-77.3	0.50 (7)	5.51	10- 4	-290 / 649	0.26 (7)	
4- 5	-1142 / 584	-77.3	-77.3	0.47 (8)	5.56	10- 5	-469 / 411	0.43 (4)	
5- 6	-1498 / 649	-77.3	-77.3	0.49 (8)	4.99	8- 5	-135 / 175	0.04 (1)	
12- 2	-1214 / 588	0.0	0.0	0.12 (1)	7.31	2-11	-490 / 1445	0.32 (1)	
7- 6	-1113 / 519	0.0	0.0	0.11 (1)	7.55	8- 6	-488 / 1370	0.31 (1)	
12-11	-183 / 156	-17.5	-17.5	0.15 (11)	6.25				
11-10	-664 / 1471	-17.5	-17.5	0.30 (1)	6.25				
10- 9	-459 / 1349	-17.5	-17.5	0.28 (1)	6.25				
9- 8	-459 / 1349	-17.5	-17.5	0.28 (1)	6.25				
8- 7	-11 / 23	-17.5	-17.5	0.14 (11)	6.25				

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.80")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.80")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.52 (2-3:7) , BC=0.30 (10-11:1) , WB=0.54 (3-10:3) , SSI=0.21 (2-3:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (8) (INPUT = 0.90)
JSI METAL= 0.48 (6) (INPUT = 1.00)

March 10, 2017

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

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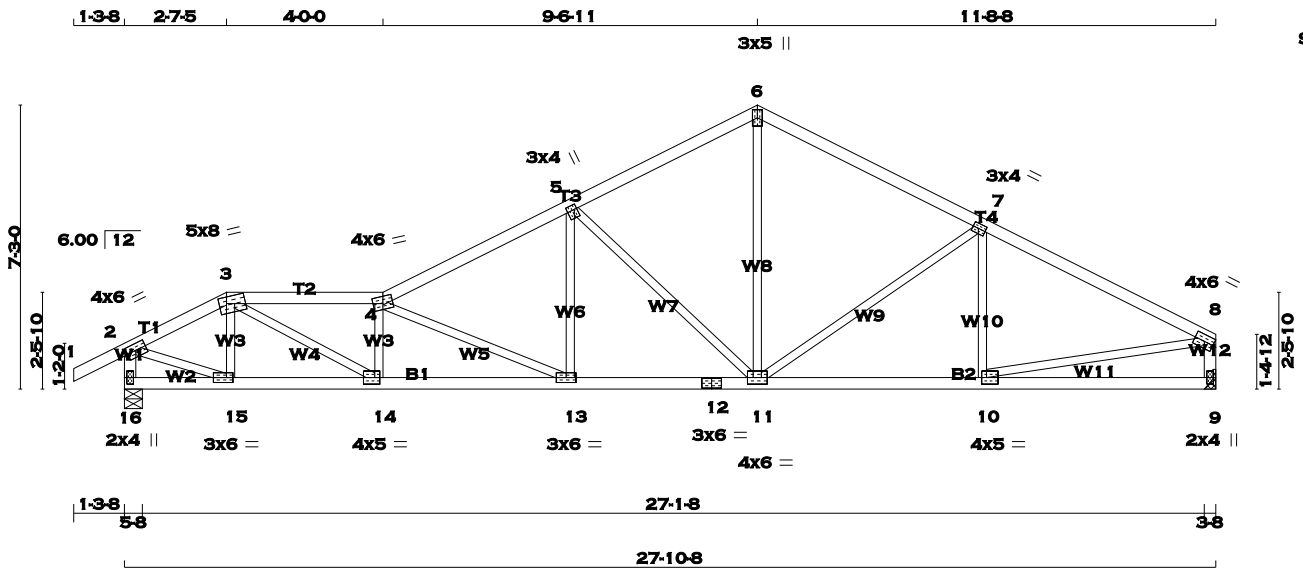

TOTAL WEIGHT = 2 X 110 = 221 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 3	2x4	DRY	No.2	SPF
3 - 4	2x4	DRY	No.2	SPF
4 - 6	2x4	DRY	No.2	SPF
6 - 8	2x4	DRY	No.2	SPF
16 - 2	2x4	DRY	No.2	SPF
9 - 8	2x4	DRY	No.2	SPF
16 - 12	2x4	DRY	No.2	SPF
12 - 9	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-t	MT20	4.0	6.0	2.00	2.75
3	TTWW-m	MT20	5.0	8.0	2.00	2.25
4	TTWW-m	MT20	4.0	6.0	2.25	3.00
5	TMWW+t	MT20	3.0	4.0	1.50	0.75
6	TTW+p	MT20	3.0	5.0		
7	TMWW-t	MT20	3.0	4.0	1.50	1.75
8	TMVW-t	MT20	4.0	6.0		Edge
9	BMV1+p	MT20	2.0	4.0		
10	BMWW-t	MT20	4.0	5.0	2.00	1.50
11	BMWWW-t	MT20	4.0	6.0		
12	BS-t	MT20	3.0	6.0		
13	BMWW-t	MT20	3.0	6.0		
14	BMWW-t	MT20	4.0	5.0	2.00	1.50
15	BMWW-t	MT20	3.0	6.0	1.50	2.00
16	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
16	1427	0	1461	201	-673	5-8	5-8
9	1322	0	1347	0	-553	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

PROVIDE ANCHORAGE AT BEARING JOINT 16 FOR 673 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 9 FOR 553 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 201 LBS FACTORED HORIZONTAL REACTION AT JOINT 16

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
16	999	712 / 0	0 / 0	0 / 0	85 / -665	287 / 0	0 / 0
9	928	649 / 0	0 / 0	0 / 0	62 / -574	279 / 0	0 / 0
HORIZONTAL REACTIONS							
16	---	0 / 0	0 / 0	0 / 0	144 / -118	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 16

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.81 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 5.42 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (11)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
1- 2	0 / 23	-77.3	-77.3	0.10 (1)	10.00
2- 3	-1697 / 782	-77.3	-77.3	0.19 (7)	5.08
3- 4	-3096 / 1394	-77.3	-77.3	0.34 (7)	3.81
4- 5	-2274 / 1022	-77.3	-77.3	0.40 (7)	4.33
5- 6	-1497 / 727	-77.3	-77.3	0.36 (7)	5.15
6- 7	-1508 / 745	-77.3	-77.3	0.49 (8)	4.99
7- 8	-1808 / 750	-77.3	-77.3	0.50 (8)	4.61
16- 2	-1447 / 687	0.0	0.0	0.14 (1)	6.84
9- 8	-1303 / 581	0.0	0.0	0.13 (1)	7.12
16-15	-183 / 156	-17.5	-17.5	0.04 (11)	6.25
15-14	-770 / 1528	-17.5	-17.5	0.28 (1)	6.25
14-13	-1452 / 3166	-17.5	-17.5	0.56 (1)	5.42
13-12	-889 / 2078	-17.5	-17.5	0.38 (1)	6.25
12-11	-889 / 2078	-17.5	-17.5	0.38 (1)	6.25
11-10	-549 / 1633	-17.5	-17.5	0.32 (1)	6.25
10- 9	-11 / 23	-17.5	-17.5	0.14 (11)	6.25

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
15- 3	-396 / 224	0.06 (1)
3-14	-757 / 1828	0.40 (1)
14- 4	-841 / 438	0.13 (1)
4-13	-1195 / 619	0.55 (1)
13- 5	-199 / 589	0.13 (1)
5-11	-1025 / 640	0.79 (3)
11- 6	-468 / 1011	0.42 (7)
11- 7	-418 / 393	0.39 (4)
10- 7	-186 / 193	0.05 (1)
2-15	-631 / 1581	0.35 (1)
10- 8	-580 / 1659	0.37 (1)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN AND TERRAIN WITH SCATTERED OBSTRUCTIONS} TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVES

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.93")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.93")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.27")

CSI: TC=0.50 (7-8:8) , BC=0.56 (13-14:1) , WB=0.79 (5-11:3) , SSI=0.20 (7-8:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (13) (INPUT = 0.90)
JSI METAL= 0.51 (12) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
T.L. WISE
100083566
PROVINCE OF ONTARIO
March 10, 2017

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

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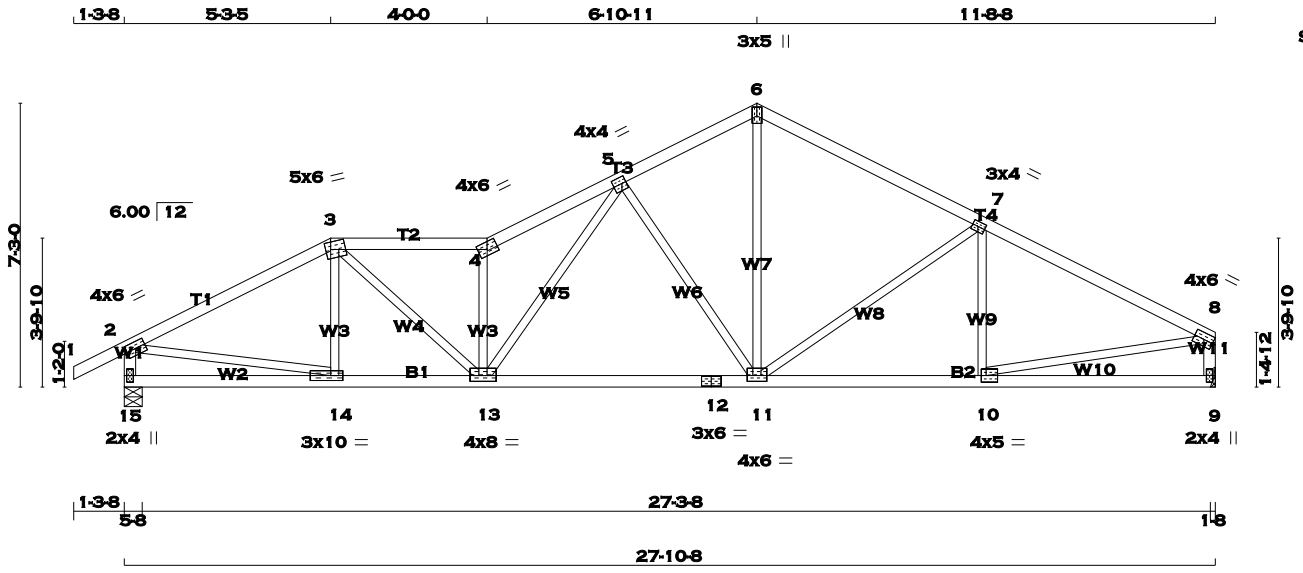


TOTAL WEIGHT = 2 X 112 = 225 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 3	2x4	DRY	No.2	SPF
3 - 4	2x4	DRY	No.2	SPF
4 - 6	2x4	DRY	No.2	SPF
6 - 8	2x4	DRY	No.2	SPF
15 - 2	2x4	DRY	No.2	SPF
9 - 8	2x4	DRY	No.2	SPF
15 - 12	2x4	DRY	No.2	SPF
12 - 9	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

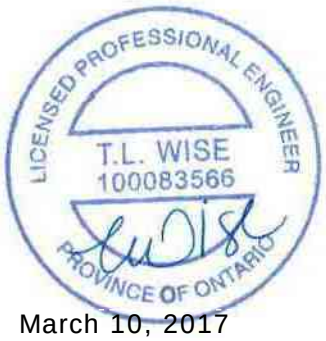
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-t	MT20	4.0	6.0	2.00	3.00
3	TTWW-m	MT20	5.0	6.0	2.25	1.50
4	TTW-h	MT20	4.0	6.0	2.25	3.25
5	TMWW-t	MT20	4.0	4.0	2.00	1.25
6	TTW+p	MT20	3.0	5.0	2.75	1.50
7	TMWW-t	MT20	3.0	4.0	1.50	1.75
8	TMVW-t	MT20	4.0	6.0		Edge
9	BMV1+p	MT20	2.0	4.0		
10	BMWW-t	MT20	4.0	5.0	2.00	1.50
11	BMWWW-t	MT20	4.0	6.0	1.75	3.00
12	BS-t	MT20	3.0	6.0		
13	BMWWW-t	MT20	4.0	8.0	1.75	4.00
14	BMWW-t	MT20	3.0	10.0	1.50	3.75
15	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.



March 10, 2017

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
15	1427	0	1461	201	-673	5-8	5-8
9	1322	0	1347	0	-553	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

PROVIDE ANCHORAGE AT BEARING JOINT 15 FOR 673 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 9 FOR 553 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 201 LBS FACTORED HORIZONTAL REACTION AT JOINT 15

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
15	999	712 / 0	0 / 0	0 / 0	85 / -665	287 / 0	0 / 0
9	928	649 / 0	0 / 0	0 / 0	62 / -574	279 / 0	0 / 0
HORIZONTAL REACTIONS							
15	---	0 / 0	0 / 0	0 / 0	144 / -118	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 15

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.13 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
1- 2	0 / 23	-77.3	-77.3	0.10 (1)	10.00
2- 3	-1871 / 838	-77.3	-77.3	0.45 (7)	4.64
3- 4	-2381 / 1101	-77.3	-77.3	0.30 (7)	4.32
4- 5	-2697 / 1306	-77.3	-77.3	0.32 (7)	4.13
5- 6	-1498 / 744	-77.3	-77.3	0.24 (7)	5.27
6- 7	-1513 / 742	-77.3	-77.3	0.49 (8)	4.98
7- 8	-1806 / 751	-77.3	-77.3	0.50 (8)	4.61
15- 2	-1420 / 701	0.0	0.0	0.14 (1)	6.89
9- 8	-1301 / 582	0.0	0.0	0.13 (1)	7.12
15-14	-183 / 156	-17.5	-17.5	0.11 (11)	6.25
14-13	-781 / 1696	-17.5	-17.5	0.30 (1)	6.25
13-12	-749 / 1820	-17.5	-17.5	0.38 (1)	6.25
12-11	-749 / 1820	-17.5	-17.5	0.38 (1)	6.25
11-10	-550 / 1631	-17.5	-17.5	0.36 (1)	6.25
10- 9	-11 / 23	-17.5	-17.5	0.13 (11)	6.25

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
14- 3	-176 / 154	0.04 (1)
3-13	-358 / 954	0.21 (1)
13- 4	-1478 / 788	0.33 (1)
13- 5	-571 / 1117	0.40 (7)
5-11	-872 / 591	0.59 (3)
11- 6	-506 / 1058	0.45 (7)
11- 7	-409 / 397	0.38 (4)
10- 7	-194 / 188	0.05 (1)
2-14	-614 / 1683	0.37 (1)
10- 8	-581 / 1657	0.37 (1)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM A VE

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

RECEIVED
TOWN OF MILTON
MAR 29, 2017
17-4978
BUILDING DIVISION

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.93")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.93")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.25")

CSI: TC=0.50 (7-8:8) , BC=0.38 (11-13:1) , WB=0.59 (5-11:3) , SSI=0.20 (7-8:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

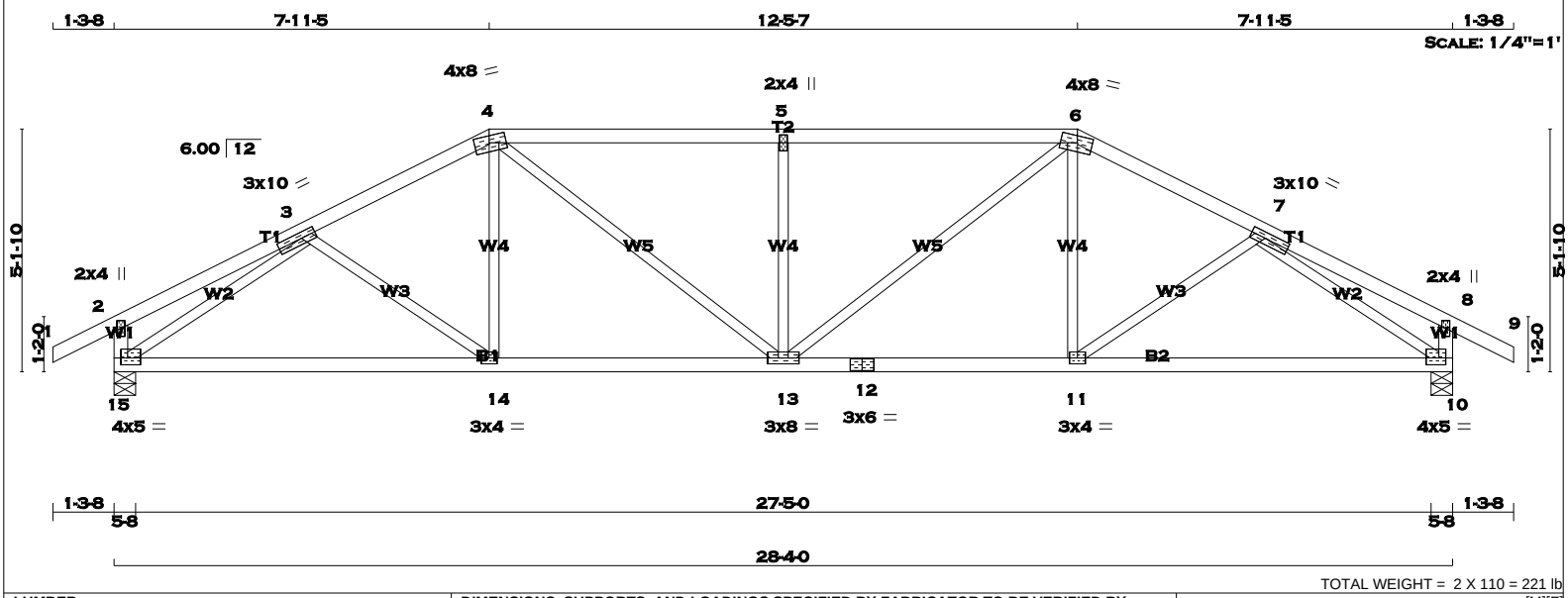
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (14) (INPUT = 0.90)
JSI METAL= 0.53 (4) (INPUT = 1.00)



TOTAL WEIGHT = 2 X 110 = 221 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 6	2x4	DRY	No.2	SPF
6 - 9	2x4	DRY	No.2	SPF
15 - 2	2x4	DRY	No.2	SPF
10 - 8	2x4	DRY	No.2	SPF
15 - 12	2x4	DRY	No.2	SPF
12 - 10	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMV+p	MT20	2.0	4.0		
3	TMWW-t	MT20	3.0	10.0	1.50	3.75
4	TTWW-m	MT20	4.0	8.0	1.75	3.75
5	TMV+w	MT20	2.0	4.0		
6	TTWW-m	MT20	4.0	8.0	1.75	3.75
7	TMWW-t	MT20	3.0	10.0	1.50	3.75
8	TMV+p	MT20	2.0	4.0		
10	BMVW1-t	MT20	4.0	5.0	1.75	1.75
11	BMWW-t	MT20	3.0	4.0		
12	BS-t	MT20	3.0	6.0		
13	BMWWW-t	MT20	3.0	8.0		
14	BMWW-t	MT20	3.0	4.0		
15	BMVW1-t	MT20	4.0	5.0	1.75	1.75

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
15	1449	0	1493	-144	-732	5-8	5-8
10	1449	0	1459	0	-732	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 15 FOR 732 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 10 FOR 732 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 144 LBS FACTORED HORIZONTAL REACTION AT JOINT 15

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
15	1014	723 / 0	0 / 0	0 / 0	112 / -710	291 / 0	0 / 0
10	1014	723 / 0	0 / 0	0 / 0	27 / -710	291 / 0	0 / 0
HORIZONTAL REACTIONS							
15	---	0 / 0	0 / 0	0 / 0	103 / -103	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 15, 10

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.28 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 23	-77.3	-77.3	0.10 (1)	10.00	3-14	-37 / 225	0.04 (7)
2- 3	-9 / 118	-77.3	-77.3	0.18 (7)	10.00	14- 4	-20 / 171	0.05 (11)
3- 4	-1879 / 972	-77.3	-77.3	0.30 (7)	4.82	4-13	-409 / 558	0.50 (8)
4- 5	-2114 / 1142	-77.3	-77.3	0.51 (7)	4.28	13- 5	-622 / 481	0.23 (3)
5- 6	-2114 / 1142	-77.3	-77.3	0.51 (7)	4.28	13- 6	-409 / 586	0.50 (7)
6- 7	-1859 / 972	-77.3	-77.3	0.30 (8)	4.83	11- 6	-21 / 177	0.05 (11)
7- 8	-9 / 118	-77.3	-77.3	0.19 (8)	10.00	11- 7	-48 / 225	0.04 (8)
8- 9	0 / 23	-77.3	-77.3	0.10 (1)	10.00	15- 3	-2064 / 962	0.78 (1)
15- 2	-235 / 213	0.0	0.0	0.03 (7)	7.81	7-10	-2046 / 962	0.78 (1)
10- 8	-234 / 213	0.0	0.0	0.03 (8)	7.81			
15-14	-887 / 1712	-17.5	-17.5	0.40 (1)	6.25			
14-13	-704 / 1686	-17.5	-17.5	0.40 (1)	6.25			
13-12	-601 / 1663	-17.5	-17.5	0.40 (1)	6.25			
12-11	-601 / 1663	-17.5	-17.5	0.40 (1)	6.25			
11-10	-766 / 1665	-17.5	-17.5	0.40 (1)	6.25			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.94")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.94")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.51 (4-5:7) , BC=0.40 (10-11:1) , WB=0.78 (3-15:1) , SSI=0.23 (4-5:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

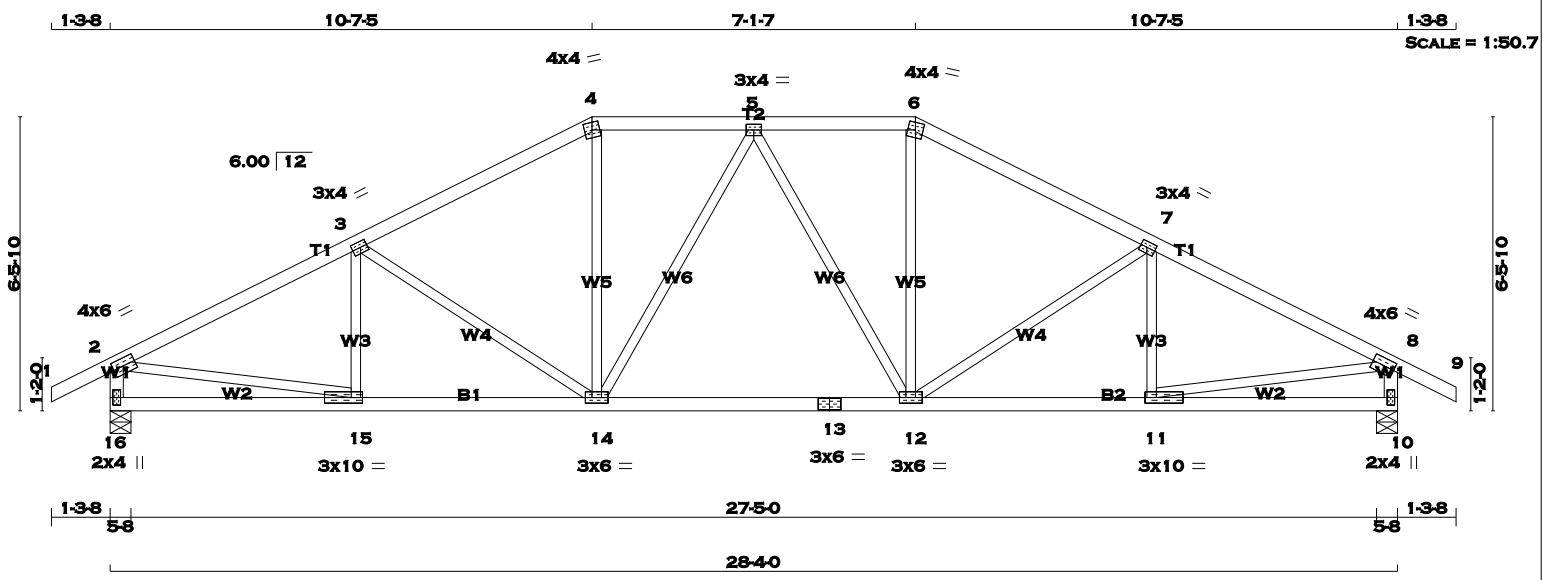
JSI GRIP= 0.89 (6) (INPUT = 0.90)
JSI METAL= 0.50 (15) (INPUT = 1.00)

March 10, 2017

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

RECEIVED
TOWN OF MILTON
MAR 29, 2017
17-4978
BUILDING DIVISION



TOTAL WEIGHT = 2 X 117 = 234 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 6	2x4	DRY	No.2	SPF
6 - 9	2x4	DRY	No.2	SPF
16 - 2	2x4	DRY	No.2	SPF
10 - 8	2x4	DRY	No.2	SPF
16 - 13	2x4	DRY	No.2	SPF
13 - 10	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-t	MT20	4.0	6.0	2.00	2.75
3	TMWW-t	MT20	3.0	4.0	1.50	1.75
4	TTW-m	MT20	4.0	4.0		
5	TMWW-t	MT20	3.0	4.0		
6	TTW-m	MT20	4.0	4.0		
7	TMWW-t	MT20	3.0	4.0	1.50	1.75
8	TMVW-t	MT20	4.0	6.0	2.00	2.75
10	BMV1+p	MT20	2.0	4.0		
11	BMWW-t	MT20	3.0	10.0	1.50	3.00
12	BMWWW-t	MT20	3.0	6.0		
13	BS-t	MT20	3.0	6.0		
14	BMWWW-t	MT20	3.0	6.0		
15	BMWW-t	MT20	3.0	10.0	1.50	3.00
16	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
16	1449	0	1487	-170	-703	5-8	5-8
10	1449	0	1467	0	-703	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 16 FOR 703 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 10 FOR 703 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 170 LBS FACTORED HORIZONTAL REACTION AT JOINT 16

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
16	1014	723 / 0	0 / 0	0 / 0	96 / -689	291 / 0	0 / 0
10	1014	723 / 0	0 / 0	0 / 0	47 / -689	291 / 0	0 / 0
HORIZONTAL REACTIONS							
16	---	0 / 0	0 / 0	0 / 0	121 / -121	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 16, 10

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.56 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 23	-77.3	-77.3	0.10 (1)	10.00	15- 3	-189 / 190	0.04 (1)
2- 3	-1972 / 942	-77.3	-77.3	0.45 (7)	4.56	3-14	-362 / 345	0.26 (3)
3- 4	-1694 / 870	-77.3	-77.3	0.44 (7)	4.86	14- 4	-206 / 473	0.14 (8)
4- 5	-1507 / 849	-77.3	-77.3	0.23 (7)	5.28	14- 5	-201 / 235	0.19 (4)
5- 6	-1490 / 849	-77.3	-77.3	0.23 (8)	5.30	5-12	-215 / 235	0.20 (3)
6- 7	-1681 / 870	-77.3	-77.3	0.44 (8)	4.86	12- 6	-205 / 478	0.14 (7)
7- 8	-1938 / 942	-77.3	-77.3	0.45 (8)	4.56	12- 7	-367 / 345	0.26 (4)
8- 9	0 / 23	-77.3	-77.3	0.10 (1)	10.00	11- 7	-186 / 190	0.04 (1)
16- 2	-1445 / 728	0.0	0.0	0.14 (1)	6.85	2-15	-727 / 1795	0.39 (1)
10- 8	-1425 / 728	0.0	0.0	0.14 (1)	6.87	11- 8	-727 / 1781	0.39 (1)
16-15	-152 / 174	-17.5	-17.5	0.10 (11)	6.25			
15-14	-858 / 1814	-17.5	-17.5	0.36 (1)	6.25			
14-13	-612 / 1609	-17.5	-17.5	0.33 (1)	6.25			
13-12	-612 / 1609	-17.5	-17.5	0.33 (1)	6.25			
12-11	-702 / 1760	-17.5	-17.5	0.36 (1)	6.25			
11-10	-9 / 18	-17.5	-17.5	0.10 (11)	10.00			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM [illegible]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.94")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.94")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.45 (7-8:8) , BC=0.36 (14-15:1) , WB=0.39 (2-15:1) , SSI=0.18 (2-3:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (11) (INPUT = 0.90)
JSI METAL= 0.53 (8) (INPUT = 1.00)

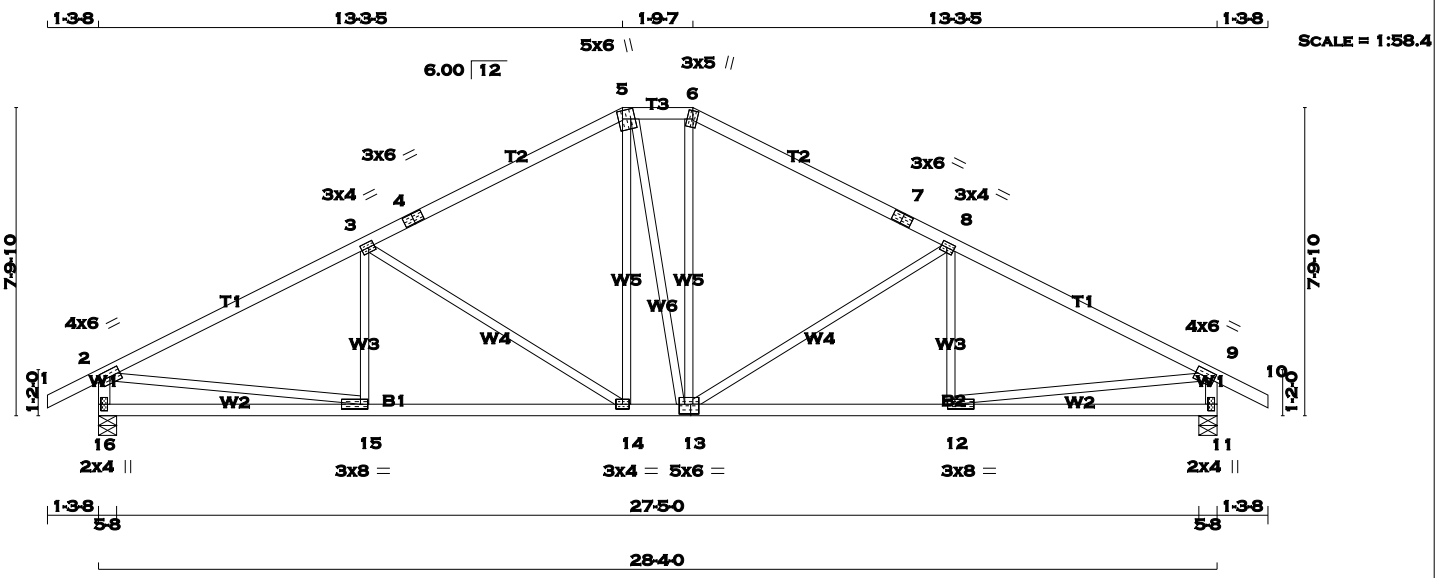
LICENSED PROFESSIONAL ENGINEER
T.L. WISE
100083566
PROVINCE OF ONTARIO

March 10, 2017

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

RECEIVED
TOWN OF MILTON
MAR 29, 2017
17-4978
BUILDING DIVISION



TOTAL WEIGHT = 2 X 120 = 241 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 5	2x4	DRY	No.2	SPF
5 - 6	2x4	DRY	No.2	SPF
6 - 7	2x4	DRY	No.2	SPF
7 - 10	2x4	DRY	No.2	SPF
16 - 2	2x4	DRY	No.2	SPF
11 - 9	2x4	DRY	No.2	SPF
16 - 13	2x4	DRY	No.2	SPF
13 - 11	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-t	MT20	4.0	6.0	2.00	2.75
3	TMWW-t	MT20	3.0	4.0	1.50	1.75
4	TS-t	MT20	3.0	6.0		
5	TTWW+m	MT20	5.0	6.0	2.50	1.25
6	TTW+m	MT20	3.0	5.0	2.50	1.25
7	TS-t	MT20	3.0	6.0		
8	TMWW-t	MT20	3.0	4.0	1.50	1.75
9	TMVW-t	MT20	4.0	6.0	2.00	2.75
11	BMV1+p	MT20	2.0	4.0		
12	BMWW-t	MT20	3.0	8.0	1.50	2.25
13	BSWWW-l	MT20	5.0	6.0	3.00	2.50
14	BMWW-t	MT20	3.0	4.0		
15	BMWW-t	MT20	3.0	8.0	1.50	2.25
16	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

March 10, 2017

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
16	1449	0	1479	-196	-665	5-8	5-8
11	1449	0	1474	0	-665	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 16 FOR 665 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 11 FOR 665 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 196 LBS FACTORED HORIZONTAL REACTION AT JOINT 16

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
16	1014	723 / 0	0 / 0	0 / 0	76 / -662	291 / 0	0 / 0
11	1014	723 / 0	0 / 0	0 / 0	64 / -662	291 / 0	0 / 0
HORIZONTAL REACTIONS							
16	---	0 / 0	0 / 0	0 / 0	140 / -140	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 16, 11

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.29 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 23	-77.3	-77.3	0.10 (1)	10.00	15- 3	-94 / 182	0.04 (7)
2- 3	-1969 / 875	-77.3	-77.3	0.63 (7)	4.29	3-14	-611 / 497	0.77 (3)
3- 4	-1467 / 728	-77.3	-77.3	0.61 (7)	4.87	14- 5	-215 / 385	0.24 (7)
4- 5	-1467 / 728	-77.3	-77.3	0.61 (7)	4.87	5-13	-110 / 103	0.13 (8)
5- 6	-1294 / 729	-77.3	-77.3	0.13 (8)	5.68	13- 6	-182 / 388	0.20 (7)
6- 7	-1471 / 728	-77.3	-77.3	0.61 (8)	4.87	13- 8	-609 / 496	0.77 (4)
7- 8	-1471 / 728	-77.3	-77.3	0.61 (8)	4.87	12- 8	-96 / 184	0.04 (8)
8- 9	-1959 / 875	-77.3	-77.3	0.63 (8)	4.29	2-15	-646 / 1789	0.52 (7)
9-10	0 / 23	-77.3	-77.3	0.10 (1)	10.00	12- 9	-646 / 1782	0.52 (8)
16- 2	-1430 / 697	0.0	0.0	0.14 (1)	6.87			
11- 9	-1425 / 696	0.0	0.0	0.14 (1)	6.88			
16-15	-178 / 208	-17.5	-17.5	0.19 (11)	6.25			
15-14	-816 / 1823	-17.5	-17.5	0.37 (1)	6.25			
14-13	-400 / 1313	-17.5	-17.5	0.27 (1)	6.25			
13-12	-624 / 1768	-17.5	-17.5	0.37 (1)	6.25			
12-11	-9 / 18	-17.5	-17.5	0.19 (11)	10.00			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM A

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.94")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.94")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.63 (8-9:8) , BC=0.37 (14-15:1) , WB=0.77 (3-14:3) , SSI=0.22 (2-3:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (15) (INPUT = 0.90)
JSI METAL= 0.53 (2) (INPUT = 1.00)

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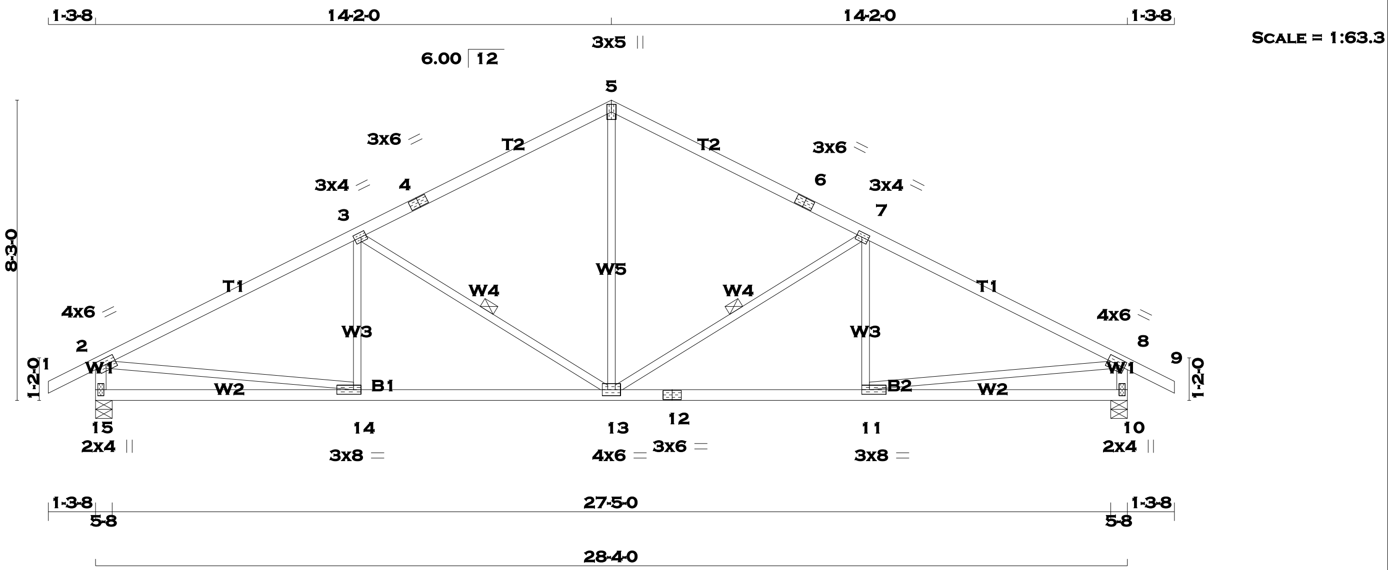


TOTAL WEIGHT = 6 X 111 = 665 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 5	2x4	DRY	No.2	SPF
5 - 6	2x4	DRY	No.2	SPF
6 - 9	2x4	DRY	No.2	SPF
15 - 2	2x4	DRY	No.2	SPF
10 - 8	2x4	DRY	No.2	SPF
15 - 12	2x4	DRY	No.2	SPF
12 - 10	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-t	MT20	4.0	6.0	2.00	2.75
3	TMWW-t	MT20	3.0	4.0	1.50	1.75
4	TS-t	MT20	3.0	6.0		
5	TTW+p	MT20	3.0	5.0		
6	TS-t	MT20	3.0	6.0		
7	TMWW-t	MT20	3.0	4.0	1.50	1.75
8	TMVW-t	MT20	4.0	6.0	2.00	2.75
10	BMV1+p	MT20	2.0	4.0		
11	BMWW-t	MT20	3.0	8.0	1.50	2.50
12	BS-t	MT20	3.0	6.0		
13	BMWWW-t	MT20	4.0	6.0		
14	BMWW-t	MT20	3.0	8.0	1.50	2.50
15	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
15	1449	0	1476	-204	-650	5-8	5-8
10	1449	0	1476	0	-650	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 15 FOR 650 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 10 FOR 650 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 204 LBS FACTORED HORIZONTAL REACTION AT JOINT 15

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
15	1014	723 / 0	0 / 0	0 / 0	69 / -652	291 / 0	0 / 0
10	1014	723 / 0	0 / 0	0 / 0	69 / -652	291 / 0	0 / 0

HORIZONTAL REACTIONS

JT		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
15	---	0 / 0	0 / 0	0 / 0	146 / -146	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 15, 10

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.17 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 7-13, 3-13. DBS = 20-0-0 . CBF = 79 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (11)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
1- 2	0 / 23	-77.3	-77.3	0.10 (1)	10.00
2- 3	-1956 / 845	-77.3	-77.3	0.70 (7)	4.17
3- 4	-1405 / 699	-77.3	-77.3	0.67 (7)	4.84
4- 5	-1405 / 699	-77.3	-77.3	0.67 (7)	4.84
5- 6	-1405 / 699	-77.3	-77.3	0.67 (8)	4.84
6- 7	-1405 / 699	-77.3	-77.3	0.67 (8)	4.84
7- 8	-1956 / 846	-77.3	-77.3	0.70 (8)	4.17
8- 9	0 / 23	-77.3	-77.3	0.10 (1)	10.00
15- 2	-1423 / 685	0.0	0.0	0.14 (1)	6.88
10- 8	-1423 / 685	0.0	0.0	0.14 (1)	6.88
15-14	-186 / 220	-17.5	-17.5	0.21 (11)	6.25
14-13	-795 / 1815	-17.5	-17.5	0.38 (1)	6.25
13-12	-592 / 1762	-17.5	-17.5	0.38 (1)	6.25
12-11	[illegible]59[illegible] / 17[illegible]2	[illegible]	[illegible]	[illegible]	6.25
11-10	[illegible] / 18[illegible]	[illegible]17[illegible]	[illegible]5	0.2[illegible]11)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
13- 5	-352 / 814	0.46 (8)
13- 7	-677 / 547	0.30 (4)
11- 7	-75 / 178	0.03 (8)
3-13	-677 / 547	0.30 (3)
14- 3	-75 / 178	0.03 (7)
2-14	-613 / 1777	0.59 (7)
11- 8	-613 / 1777	0.59 (8)

KOTT

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF {9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COE[illegible]STING SYSTEM}.INTERNAL WIN[illegible]G MAY BE LOCATED ON {OPE[illegible] LEAST {0-0} FT-IN-SX AWAY FRO[illegible]

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.94")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.94")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.70 (7-8:8) , BC=0.38 (13-14:1) , WB=0.59 (8-11:8) , SSI=0.24 (2-3:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

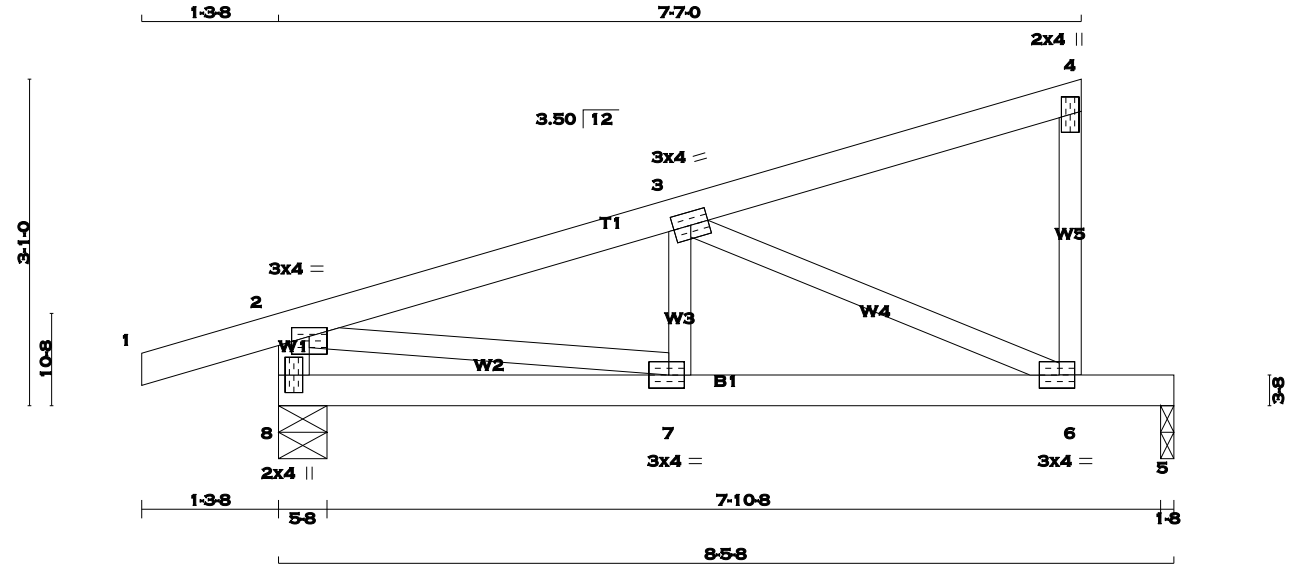
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (11) (INPUT = 0.90)
JSI METAL= 0.53 (8) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
T.L. WISE
100083566
PROVINCE OF ONTARIO

March 10, 2017



TOTAL WEIGHT = 16 X 30 = 475 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
6 - 4	2x3	DRY	No.2	SPF
8 - 2	2x4	DRY	No.2	SPF
8 - 5	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-p	MT20	3.0	4.0	1.00	2.00
3	TMWW-t	MT20	3.0	4.0		
4	TMV+p	MT20	2.0	4.0		
6	BMVW-t	MT20	3.0	4.0	1.50	1.75
7	BMWW-t	MT20	3.0	4.0	1.50	1.75
8	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
8	501	0	501	203	-261	5-8	5-8
5	337	0	337	0	-174	1-8	1-8

PROVIDE ANCHORAGE AT BEARING JOINT 8 FOR 261 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 5 FOR 174 LBS FACTORED UPLIFT

PROVIDE FOR 203 LBS FACTORED HORIZONTAL REACTION AT JOINT 8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
8	349	257 / 0	0 / 0	0 / 0	0 / -246	92 / 0	0 / 0
5	238	158 / 0	0 / 0	0 / 0	0 / -175	80 / 0	0 / 0
HORIZONTAL REACTIONS							
8	---	0 / 0	0 / 0	0 / 0	145 / 0	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 8, 5

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 14	-77.3	-77.3	0.10 (1)	10.00	7- 3	0 / 97	0.03 (11)
2- 3	-630 / 326	-77.3	-77.3	0.18 (5)	6.25	3- 6	-669 / 413	0.19 (1)
3- 4	-53 / 38	-77.3	-77.3	0.14 (5)	6.25	2- 7	-272 / 626	0.14 (1)
6- 4	-115 / 84	0.0	0.0	0.12 (5)	7.81			
8- 2	-489 / 286	0.0	0.0	0.05 (1)	7.81			
8- 7	-190 / 0	-17.5	-17.5	0.11 (1)	6.25			
7- 6	-323 / 611	-17.5	-17.5	0.49 (1)	6.25			
6- 5	0 / 0	-17.5	-17.5	0.39 (1)	10.00			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.28")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.28")
CALCULATED VERT. DEFL.(TL) = L/ 874 (0.12")

CSI: TC=0.18 (2-3:5) , BC=0.49 (6-7:1) , WB=0.19 (3-6:1) , SSI=0.26 (5-6:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (7) (INPUT = 0.90)
JSI METAL= 0.23 (7) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
T.L. WISE
100083566
PROVINCE OF ONTARIO
March 10, 2017

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

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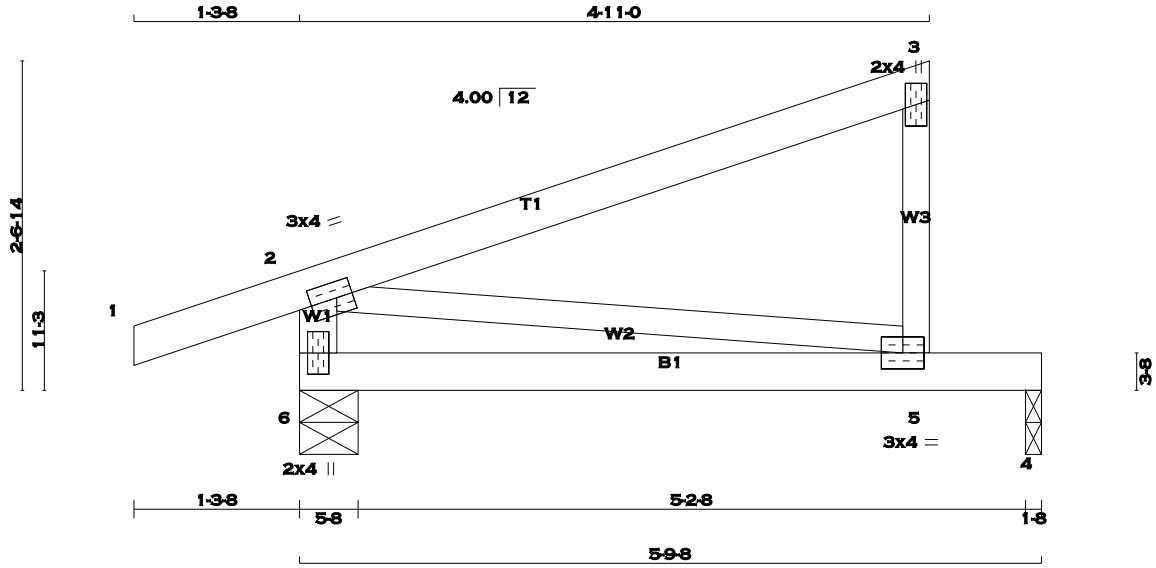
MAR 29, 2017
17-4978
BUILDING DIVISION



TOTAL WEIGHT = 14 X 20 = 278 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 3	2x4	DRY	No.2	SPF
5 - 3	2x3	DRY	No.2	SPF
6 - 2	2x4	DRY	No.2	SPF
6 - 4	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-t	MT20	3.0	4.0	1.50	1.50
3	TMV+p	MT20	2.0	4.0		
5	BMVW-t	MT20	3.0	4.0		
6	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
6	373	0	373	175	-194	5-8	5-8
4	212	0	212	0	-109	1-8	1-8

PROVIDE ANCHORAGE AT BEARING JOINT 6 FOR 194 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 4 FOR 150 LBS FACTORED UPLIFT

PROVIDE FOR 175 LBS FACTORED HORIZONTAL REACTION AT JOINT 6

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
6	260	194 / 0	0 / 0	0 / 0	0 / -181	66 / 0	0 / 0
4	150	97 / 0	0 / 0	0 / 0	0 / -112	53 / 0	0 / 0
HORIZONTAL REACTIONS							
6	---	0 / 0	0 / 0	0 / 0	125 / 0	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 6, 4

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 16	-77.3	-77.3	0.10 (1)	10.00	2- 5	0 / 136	0.03 (6)
2- 3	-69 / 11	-77.3	-77.3	0.32 (1)	6.25			
5- 3	-190 / 158	0.0	0.0	0.10 (5)	7.81			
6- 2	-294 / 206	0.0	0.0	0.03 (7)	7.81			
6- 5	-161 / 0	-17.5	-17.5	0.25 (1)	6.25			
5- 4	0 / 0	-17.5	-17.5	0.24 (1)	10.00			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	23.3	PSF
		DL =	3.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.0	PSF
TOTAL	LOAD	=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 586 (0.12")

CSI: TC=0.32 (2-3:1) , BC=0.25 (5-6:1) , WB=0.03 (2-5:6) , SSI=0.17 (4-5:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.29 (2) (INPUT = 0.90)
JSI METAL= 0.08 (6) (INPUT = 1.00)

March 10, 2017

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

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MAR 29, 2017
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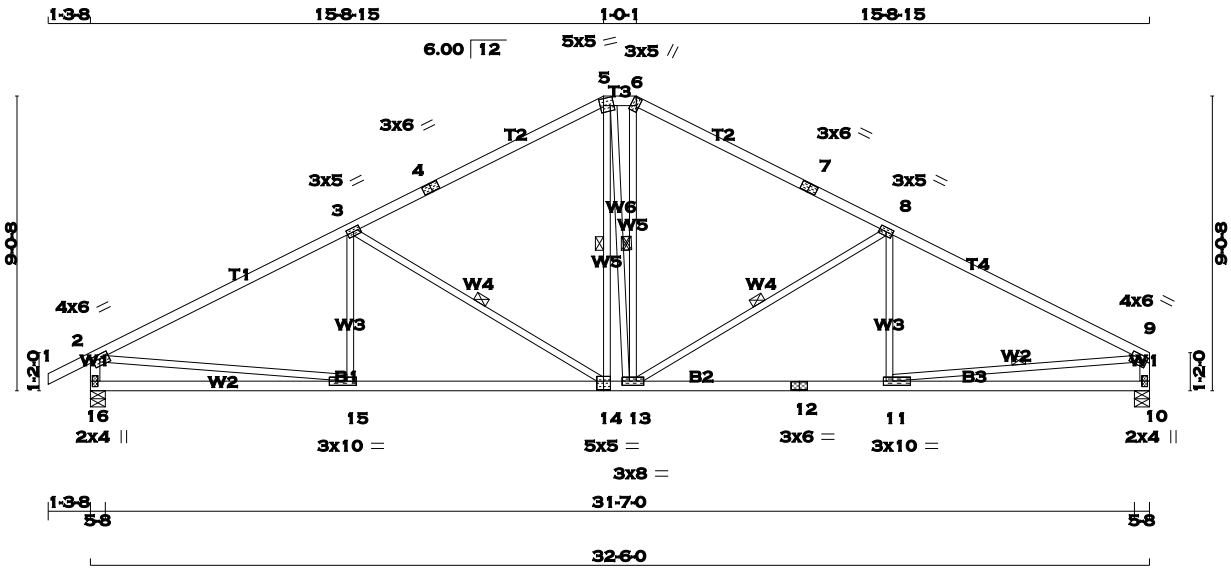

TOTAL WEIGHT = 137 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 5	2x4	DRY	No.2	SPF
5 - 6	2x4	DRY	No.2	SPF
6 - 7	2x4	DRY	No.2	SPF
7 - 9	2x4	DRY	No.2	SPF
16 - 2	2x4	DRY	No.2	SPF
10 - 9	2x4	DRY	No.2	SPF
16 - 14	2x4	DRY	No.2	SPF
14 - 12	2x4	DRY	No.2	SPF
12 - 10	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-t	MT20	4.0	6.0	1.50	2.75
3	TMWW-t	MT20	3.0	5.0		
4	TS-t	MT20	3.0	6.0		
5	TTWW-m	MT20	5.0	5.0	2.50	1.25
6	TTW+h	MT20	3.0	5.0	2.75	1.00
7	TS-t	MT20	3.0	6.0		
8	TMWW-t	MT20	3.0	5.0		
9	TMVW-t	MT20	4.0	6.0	1.50	2.75
10	BMV1+p	MT20	2.0	4.0		
11	BMWW-t	MT20	3.0	10.0	1.50	3.50
12	BS-t	MT20	3.0	6.0		
13	BMWWW-t	MT20	3.0	8.0		
14	BSWW-l	MT20	5.0	5.0	3.25	2.50
15	BMWW-t	MT20	3.0	10.0	1.50	3.50
16	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
16	1646	0	1680	242	-745	5-8	5-8
10	1541	0	1576	0	-683	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 16 FOR 745 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 10 FOR 683 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 242 LBS FACTORED HORIZONTAL REACTION AT JOINT 16

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
16	1153	820 / 0	0 / 0	0 / 0	85 / -747	333 / 0	0 / 0
10	1082	757 / 0	0 / 0	0 / 0	87 / -697	325 / 0	0 / 0
HORIZONTAL REACTIONS							
16	---	0 / 0	0 / 0	0 / 0	173 / -143	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 16, 10

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.56 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 3-14, 5-14, 5-13, 6-13, 8-13. DBS = 20-0-0 . CBF = 91 LBS.
1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 9-11. DBS = 14-0-0 . CBF = 90 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD FROM	TO	LC1 MAX (PLF)	CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED MAX CSI (LC)
1- 2	0 / 23	-77.3	-77.3	0.10 (1)		10.00	15- 3	-67 / 196	0.04 (11)
2- 3	-2315 / 1009	-77.3	-77.3	0.86 (1)		3.56	3-14	-777 / 615	0.45 (3)
3- 4	-1667 / 821	-77.3	-77.3	0.83 (7)		4.21	14- 5	-250 / 440	0.13 (7)
4- 5	-1667 / 821	-77.3	-77.3	0.83 (7)		4.21	5-13	-128 / 107	0.07 (8)
5- 6	-1468 / 811	-77.3	-77.3	0.12 (8)		5.43	13- 6	-232 / 472	0.12 (7)
6- 7	-1671 / 822	-77.3	-77.3	0.83 (8)		4.21	13- 8	-775 / 611	0.45 (4)
7- 8	-1671 / 822	-77.3	-77.3	0.83 (8)		4.21	11- 8	-72 / 202	0.04 (8)
8- 9	-2308 / 1004	-77.3	-77.3	0.86 (1)		3.56	2-15	-750 / 2100	0.97 (7)
16- 2	-1623 / 784	0.0	0.0	0.16 (1)		6.54	11- 9	-781 / 2092	0.46 (1)
10- 9	-1518 / 721	0.0	0.0	0.15 (1)		6.71			
16-15	-225 / 205	-17.5	-17.5	0.27 (11)		6.25			
15-14	-970 / 2138	-17.5	-17.5	0.47 (1)		6.25			
14-13	-44 / 1	-17	-1	0		6.25			
13-12	/ 2	-17	-1	0.4	1)	6.25			
12-11	/ 2	-17	-1	0.4	1)	6.25			
11-10	/ 1	-17	-1	0.2	11)	10.00			

WIND
{40-0
COE
WIN
{OPE
FRO
PRESSURE OF { 9.0} PSF AT
EXTERIAL PEN
SING SYSTEM}.INTERNAL
MAY BE LOCATED ON
LEAST {0-0} FT-IN-SX AWAY

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.08")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (1.08")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.25")

CSI: TC=0.86 (2-3:1) , BC=0.47 (14-15:1) , WB=0.97 (2-15:7) , SSI=0.27 (2-3:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (14) (INPUT = 0.90)
JSI METAL= 0.62 (2) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
T.L. WISE
100083566
PROVINCE OF ONTARIO
March 10, 2017

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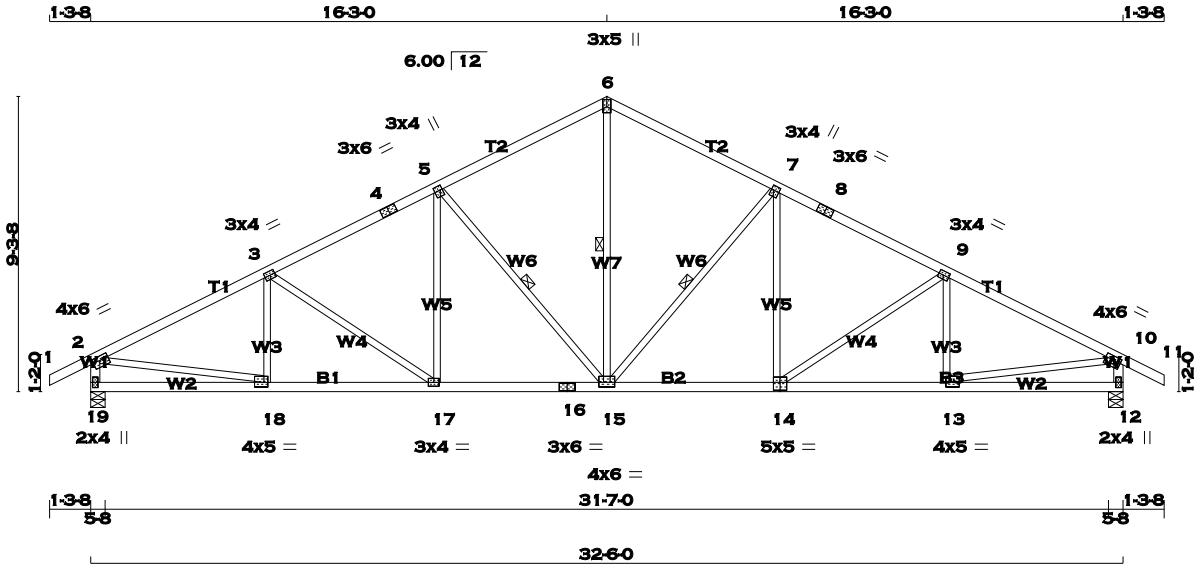


TOTAL WEIGHT = 15 X 138 = 2063 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 6	2x4	DRY	No.2	SPF
6 - 8	2x4	DRY	No.2	SPF
8 - 11	2x4	DRY	No.2	SPF
19 - 2	2x4	DRY	No.2	SPF
12 - 10	2x4	DRY	No.2	SPF
19 - 16	2x4	DRY	No.2	SPF
16 - 14	2x4	DRY	No.2	SPF
14 - 12	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-t	MT20	4.0	6.0	1.50	2.75
3	TMWW-t	MT20	3.0	4.0	1.50	1.75
4	TS-t	MT20	3.0	6.0		
5	TMWW+t	MT20	3.0	4.0	1.75	0.75
6	TTW+p	MT20	3.0	5.0	2.75	1.50
7	TMWW+t	MT20	3.0	4.0	1.75	0.75
8	TS-t	MT20	3.0	6.0		
9	TMWW-t	MT20	3.0	4.0	1.50	1.75
10	TMVW-t	MT20	4.0	6.0	1.50	2.75
12	BMV1+p	MT20	2.0	4.0		
13	BMWW-t	MT20	4.0	5.0	1.75	1.50
14	BSWW-l	MT20	5.0	5.0	3.00	2.50
15	BMWWW-t	MT20	4.0	6.0	1.75	3.00
16	BS-t	MT20	3.0	6.0		
17	BMWW-t	MT20	3.0	4.0		
18	BMWW-t	MT20	4.0	5.0	1.75	1.50
19	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

March 10, 2017

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
19	1646	0	1678	230	-738	5-8	5-8
12	1646	0	1678	0	-738	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 19 FOR 738 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 12 FOR 738 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 230 LBS FACTORED HORIZONTAL REACTION AT JOINT 19

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
19	1153	820 / 0	0 / 0	0 / 0	81 / -741	333 / 0	0 / 0
12	1153	820 / 0	0 / 0	0 / 0	81 / -741	333 / 0	0 / 0
HORIZONTAL REACTIONS							
19	---	0 / 0	0 / 0	0 / 0	165 / -165	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 19, 12

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 6-15, 7-15, 5-15. DBS = 20-0-0 . CBF = 79 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 23	-77.3	-77.3	0.10 (1)	10.00	15- 6	-515 / 1082	0.28 (8)
2- 3	-2290 / 990	-77.3	-77.3	0.41 (7)	4.25	15- 7	-673 / 518	0.30 (4)
3- 4	-2060 / 952	-77.3	-77.3	0.41 (7)	4.50	14- 7	-98 / 249	0.07 (8)
4- 5	-2060 / 952	-77.3	-77.3	0.41 (7)	4.50	14- 9	-282 / 287	0.21 (4)
5- 6	-1611 / 830	-77.3	-77.3	0.37 (7)	4.96	13- 9	-216 / 198	0.05 (1)
6- 7	-1611 / 830	-77.3	-77.3	0.37 (8)	4.96	5-15	-673 / 518	0.30 (3)
7- 8	-2060 / 952	-77.3	-77.3	0.41 (8)	4.50	17- 5	-97 / 249	0.07 (7)
8- 9	-2060 / 952	-77.3	-77.3	0.41 (8)	4.50	3-17	-282 / 287	0.21 (3)
9-10	-2290 / 990	-77.3	-77.3	0.41 (8)	4.25	18- 3	-216 / 198	0.05 (1)
10-11	0 / 23	-77.3	-77.3	0.10 (1)	10.00	2-18	-764 / 2078	0.46 (1)
19- 2	-1636 / 764	0.0	0.0	0.16 (1)	6.52	13-10	-765 / 2078	0.46 (1)
12-10	-1636 / 764	0.0	0.0	0.16 (1)	6.52			
19-18	-213 / 247	-17.5	-17.5	0.12 (11)	6.25			
18-17	-969 / 2110	-17.5	-17.5	0.37 (1)	6.25			
17-16	-732 / 1877	-17.5	-17.5	0.35 (1)	6.25			
16-15	-732 / 1877	-17.5	-17.5	0.35 (1)	6.25			
15-14	-556 / 1839	-17.5	-17.5	0.35 (1)	6.25			
14-13	-739 / 2046	-17.5	-17.5	0.37 (1)	6.25			
13-12	-9 / 18	-17.5	-17.5	0.12 (11)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.08")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (1.08")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.41 (9-10:8) , BC=0.37 (13-14:1) , WB=0.46 (10-13:1) , SSI=0.18 (9-10:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (13) (INPUT = 0.90)
JSI METAL= 0.62 (18) (INPUT = 1.00)

CONTINUED ON PAGE 2

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

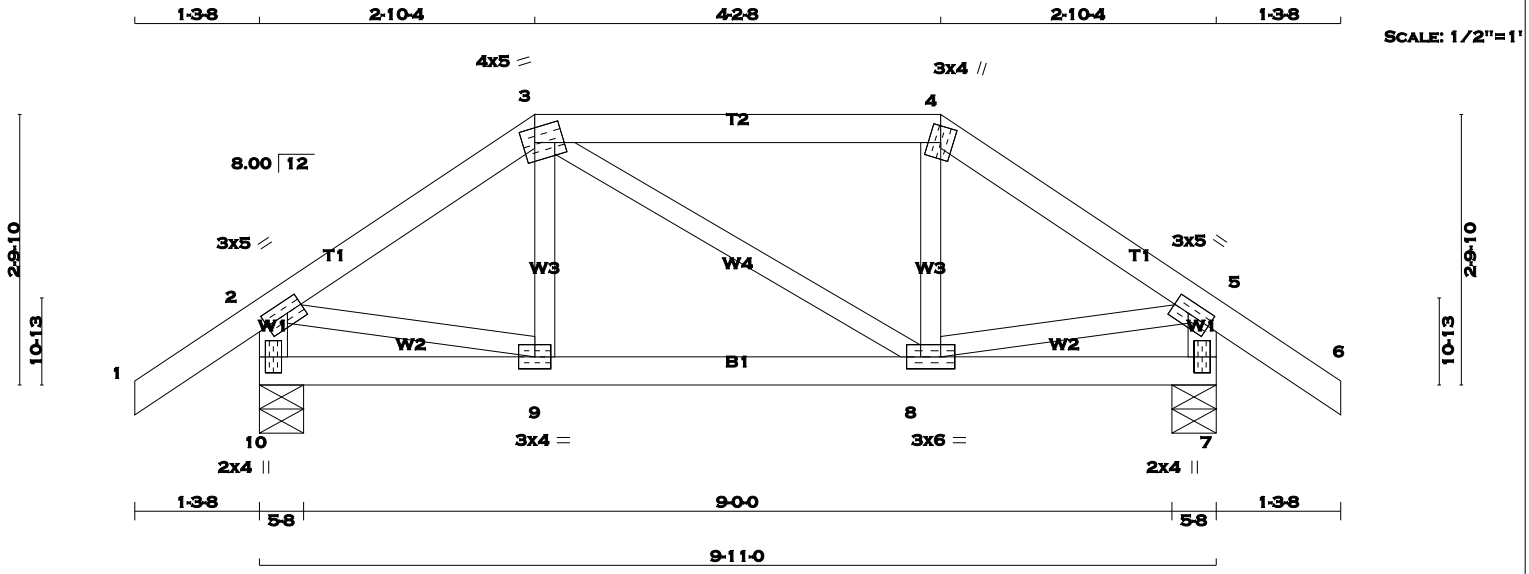
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
TW0317-048	T35	1	1	TR-GREENPARK-LECCO RIDGE-BLOCK 327	Page 94 of 159 TW0317-048

TRUSS DESC.

Kott Lumber Uxbridge, Stouffville, ON, TW

Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:20:49 2017 Page 1

ID:4BORDhR74Ei?Dog_xdfUkJyKZi_-hnrRA3iQIHkQDcPpF9ucfNk40YloVAHLaOvzyrzcJJi



TOTAL WEIGHT = 40 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 3	2x4	DRY	No.2	SPF
3 - 4	2x4	DRY	No.2	SPF
4 - 6	2x4	DRY	No.2	SPF
10 - 2	2x4	DRY	No.2	SPF
7 - 5	2x4	DRY	No.2	SPF
10 - 7	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-t	MT20	3.0	5.0	1.50	2.00
3	TTWW-m	MT20	4.0	5.0	1.75	1.50
4	TTW+m	MT20	3.0	4.0		
5	TMVW-t	MT20	3.0	5.0	1.50	2.00
7	BMV1+p	MT20	2.0	4.0		
8	BMWWW-t	MT20	3.0	6.0		
9	BMWW-t	MT20	3.0	4.0		
10	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
10	576	0	611	-145	-293	5-8	5-8
7	576	0	599	0	-293	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 10 FOR 293 LBS FACTORED UPLIFT

PROVIDE ANCHORAGE AT BEARING JOINT 7 FOR 293 LBS FACTORED UPLIFT

PROVIDE FOR 145 LBS FACTORED HORIZONTAL REACTION AT JOINT 10

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
10	402	295 / 0	0 / 0	0 / 0	87 / -278	107 / 0	0 / 0
7	402	295 / 0	0 / 0	0 / 0	57 / -278	107 / 0	0 / 0
HORIZONTAL REACTIONS							
10	---	0 / 0	0 / 0	0 / 0	103 / -103	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 10, 7

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (11)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
1- 2	0 / 29	-77.3	-77.3	0.10 (1)	10.00
2- 3	-496 / 244	-77.3	-77.3	0.15 (7)	6.25
3- 4	-409 / 262	-77.3	-77.3	0.20 (8)	6.25
4- 5	-494 / 245	-77.3	-77.3	0.15 (8)	6.25
5- 6	0 / 29	-77.3	-77.3	0.10 (1)	10.00
10- 2	-590 / 305	0.0	0.0	0.06 (1)	7.81
7- 5	-578 / 306	0.0	0.0	0.06 (1)	7.81
10- 9	-133 / 139	-17.5	-17.5	0.05 (11)	6.25
9- 8	-138 / 430	-17.5	-17.5	0.09 (1)	6.25
8- 7	-5 / 11	-17.5	-17.5	0.05 (11)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
9- 3	-13 / 71	0.02 (11)
3- 8	-55 / 59	0.02 (5)
8- 4	-16 / 85	0.02 (11)
2- 9	-122 / 410	0.09 (1)
8- 5	-123 / 425	0.09 (1)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.33")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.33")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.20 (3-4:8) , BC=0.09 (8-9:1) , WB=0.09 (5-8:1) , SSI=0.13 (3-4:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (8) (INPUT = 0.90)
JSI METAL= 0.18 (2) (INPUT = 1.00)

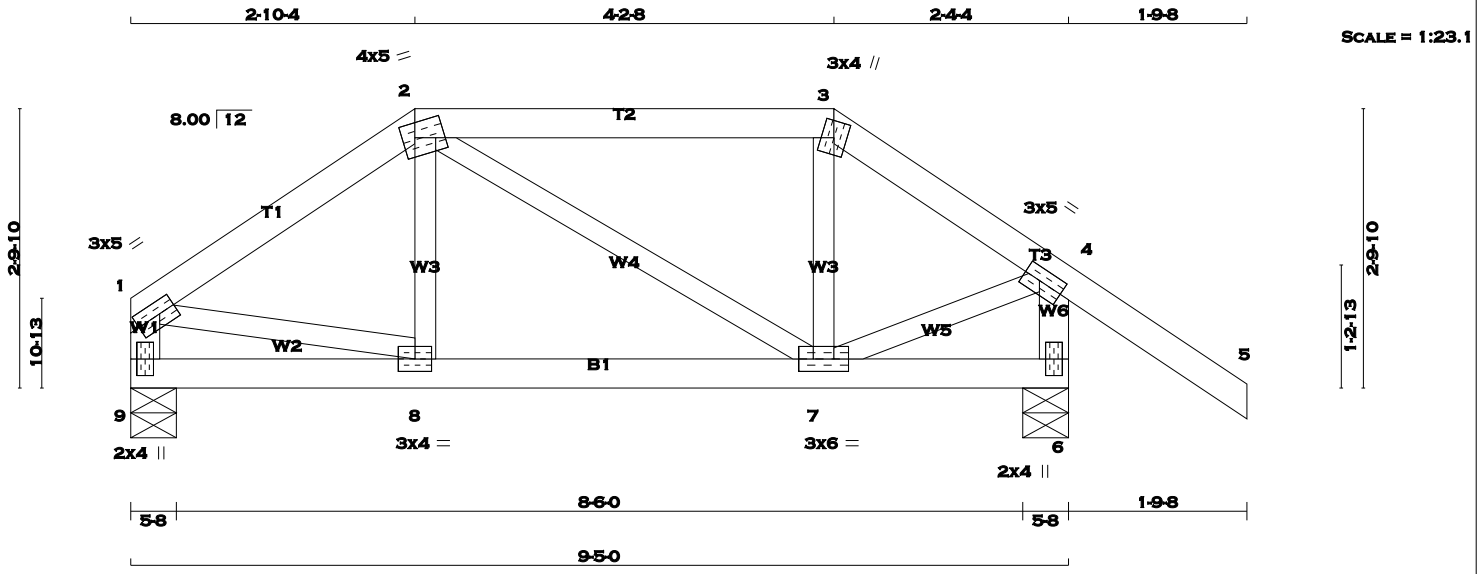
LICENSED PROFESSIONAL ENGINEER
T.L. WISE
100083566
PROVINCE OF ONTARIO

March 10, 2017

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

RECEIVED
TOWN OF MILTON
MAR 29, 2017
17-4978
BUILDING DIVISION



TOTAL WEIGHT = 38 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 2	2x4	DRY	No.2	SPF
2 - 3	2x4	DRY	No.2	SPF
3 - 5	2x4	DRY	No.2	SPF
9 - 1	2x4	DRY	No.2	SPF
6 - 4	2x4	DRY	No.2	SPF
9 - 6	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
1	TMVW-t	MT20	3.0	5.0	1.50	2.00
2	TTWW-m	MT20	4.0	5.0	1.75	1.50
3	TTW+m	MT20	3.0	4.0		
4	TMVW-t	MT20	3.0	5.0	1.50	2.00
6	BMV1+p	MT20	2.0	4.0		
7	BMWWW-t	MT20	3.0	6.0		
8	BMWW-t	MT20	3.0	4.0		
9	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
9	432	0	463	-140	-217	5-8	5-8
6	606	0	627	0	-311	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 9 FOR 217 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 6 FOR 311 LBS FACTORED UPLIFT

PROVIDE FOR 140 LBS FACTORED HORIZONTAL REACTION AT JOINT 9

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
9	304	211 / 0	0 / 0	0 / 0	76 / -215	93 / 0	0 / 0
6	422	315 / 0	0 / 0	0 / 0	54 / -291	107 / 0	0 / 0
HORIZONTAL REACTIONS							
9	---	0 / 0	0 / 0	0 / 0	94 / -100	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 9, 6

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	-444 / 215	-77.3	-77.3	0.14 (7)	6.25	8- 2	-3 / 72	0.02 (11)
2- 3	-303 / 202	-77.3	-77.3	0.19 (8)	6.25	2- 7	-88 / 59	0.03 (3)
3- 4	-394 / 211	-77.3	-77.3	0.18 (8)	6.25	7- 3	-7 / 52	0.02 (11)
4- 5	0 / 40	-77.3	-77.3	0.19 (1)	10.00	1- 8	-116 / 361	0.08 (1)
9- 1	-442 / 229	0.0	0.0	0.04 (1)	7.81	7- 4	-97 / 335	0.07 (1)
6- 4	-612 / 320	0.0	0.0	0.06 (1)	7.81			
9- 8	-121 / 135	-17.5	-17.5	0.05 (11)	6.25			
8- 7	-143 / 389	-17.5	-17.5	0.08 (1)	6.25			
7- 6	-8 / 18	-17.5	-17.5	0.04 (11)	10.00			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	23.3	PSF
		DL =	3.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.0	PSF
TOTAL	LOAD	=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.31")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.31")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.19 (2-3:8) , BC=0.08 (7-8:1) , WB=0.08 (1-8:1) , SSI=0.13 (2-3:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

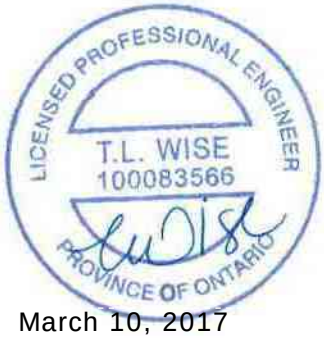
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

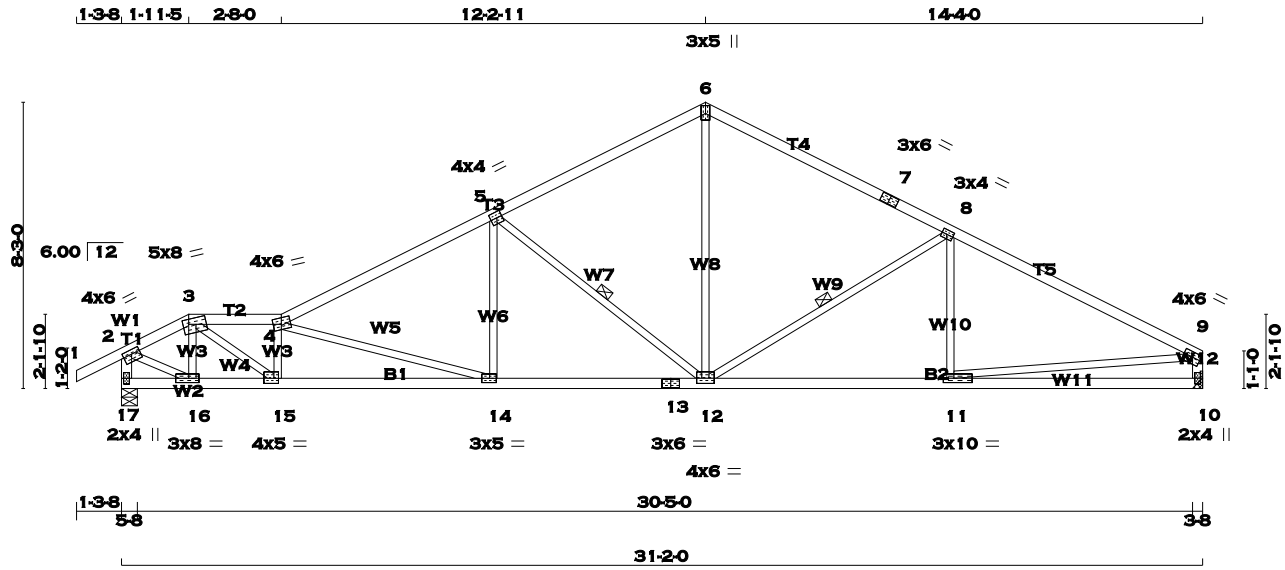
JSI GRIP= 0.64 (3) (INPUT = 0.90)
JSI METAL= 0.16 (4) (INPUT = 1.00)



March 10, 2017

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



TOTAL WEIGHT = 121 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 3	2x4	DRY	No.2	SPF
3 - 4	2x4	DRY	No.2	SPF
4 - 6	2x4	DRY	No.2	SPF
6 - 7	2x4	DRY	No.2	SPF
7 - 9	2x4	DRY	No.2	SPF
17 - 2	2x4	DRY	No.2	SPF
10 - 9	2x4	DRY	No.2	SPF
17 - 13	2x4	DRY	No.2	SPF
13 - 10	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-t	MT20	4.0	6.0	1.50	3.00
3	TTWW-m	MT20	5.0	8.0	1.75	2.00
4	TTWW-m	MT20	4.0	6.0	2.25	2.75
5	TMWW-t	MT20	4.0	4.0	2.00	1.75
6	TTW+p	MT20	3.0	5.0	2.75	1.50
7	TS-t	MT20	3.0	6.0		
8	TMWW-t	MT20	3.0	4.0	1.50	1.75
9	TMVW-t	MT20	4.0	6.0	1.75	2.75
10	BMV1+p	MT20	2.0	4.0		
11	BMWW-t	MT20	3.0	10.0	1.50	3.75
12	BMWWW-t	MT20	4.0	6.0	1.75	3.00
13	BS-t	MT20	3.0	6.0		
14	BMWW-t	MT20	3.0	5.0	1.50	2.25
15	BMWW-t	MT20	4.0	5.0	1.75	1.50
16	BMWW-t	MT20	3.0	8.0	1.50	3.50
17	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
17	1583	0	1618	221	-730	IN-SX	5-8
10	1478	0	1510	0	-635	HANGER BY OTHERS	IN-SX
						MIN. SEAT SIZE: 3-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 17 FOR 730 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 10 FOR 635 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 221 LBS FACTORED HORIZONTAL REACTION AT JOINT 17

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
17	1109	789 / 0	0 / 0	0 / 0	88 / -727	320 / 0	0 / 0
10	1037	726 / 0	0 / 0	0 / 0	79 / -654	312 / 0	0 / 0
HORIZONTAL REACTIONS							
17	---	0 / 0	0 / 0	0 / 0	158 / -132	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 17

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.87 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 5.33 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 5-12. DBS = 14-0-0 . CBF = 85 LBS.
1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 8-12. DBS = 20-0-0 . CBF = 80 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
1- 2	0 / 23	-77.3	-77.3	0.10 (1)	10.00
2- 3	-1734 / 796	-77.3	-77.3	0.16 (7)	5.07
3- 4	-3159 / 1392	-77.3	-77.3	0.26 (7)	3.87
4- 5	-2496 / 1098	-77.3	-77.3	0.58 (7)	3.96
5- 6	-1671 / 797	-77.3	-77.3	0.54 (7)	4.72
6- 7	-1682 / 816	-77.3	-77.3	0.70 (8)	4.47
7- 8	-1682 / 816	-77.3	-77.3	0.70 (8)	4.47
8- 9	-2240 / 938	-77.3	-77.3	0.73 (8)	3.89
17- 2	-1607 / 745	0.0	0.0	0.16 (1)	6.57
10- 9	-1457 / 670	0.0	0.0	0.14 (1)	6.82
17-16	-204 / 187	-17.5	-17.5	0.04 (8)	6.25
16-15	-814 / 1553	-17.5	-17.5	0.27 (1)	6.25
15-14	-50[illegible] / 3[illegible]	[illegible]	[illegible]	[illegible]	5.33
14-13	[illegible] / 2[illegible]	17	[illegible]5	0.4[illegible] (1)	6.25
13-12	[illegible] / 2[illegible]	17	[illegible]5	0.4[illegible] (1)	6.25
12-11	-71[illegible] / 2[illegible]	17	[illegible]5	0.4[illegible] (1)	6.25
11-10	-8[illegible] / 16[illegible]	[illegible]	-17.5	0.22 (11)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
16- 3	-585 / 261	0.09 (1)
3-15	-815 / 2021	0.45 (1)
15- 4	-1132 / 561	0.17 (1)
4-14	-1014 / 544	0.76 (3)
14- 5	-82 / 413	0.09 (1)
5-12	-1026 / 679	0.40 (3)
12- 6	-479 / 1083	0.62 (7)
12- 8	-687 / 552	0.31 (4)
11- 8	-77 / 185	0.04 (8)
2-16	-678 / 1671	0.37 (1)
11- 9	-732 / 2029	0.73 (8)

WIND [illegible] PRESSURE (20) PSF AT
{40-0 [illegible] EXTERNAL PEAK
COE [illegible] STING SYSTEM; INTERNAL
WIN [illegible] MAY BE LOCATED ON
{OPE [illegible] LEAST {0-0} FROM SX AWAY
FRO [illegible]

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.04")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (1.04")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.30")

CSI: TC=0.73 (8-9:8) , BC=0.60 (14-15:1) , WB=0.76 (4-14:3) , SSI=0.24 (8-9:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

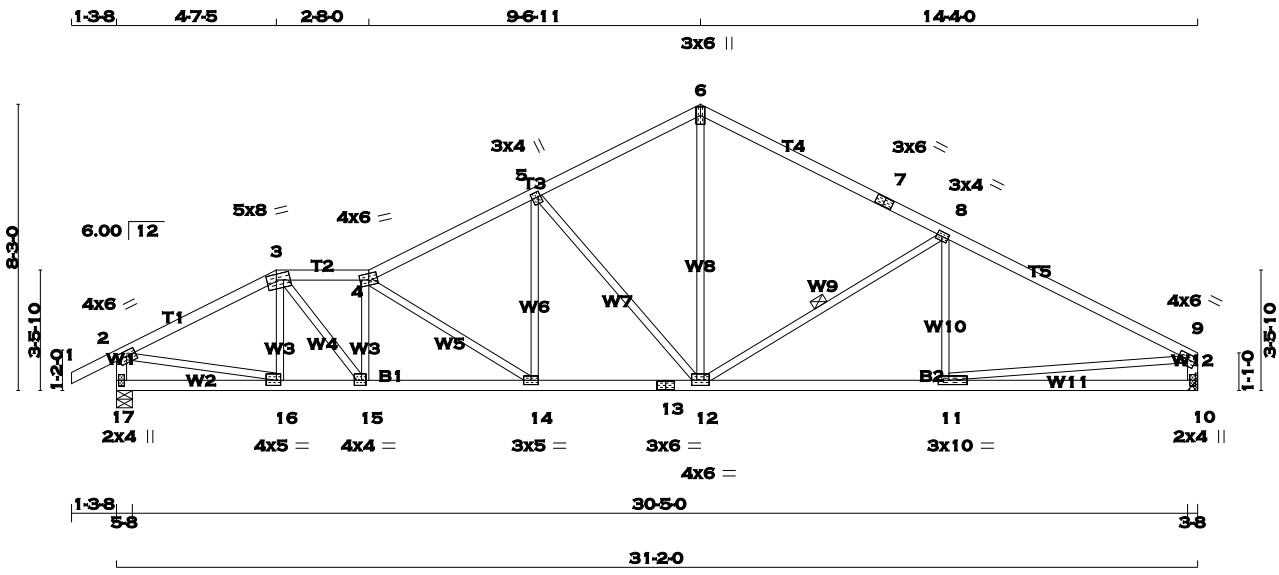
JSI GRIP= 0.90 (15) (INPUT = 0.90)
JSI METAL= 0.60 (13) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
T.L. WISE
100083566
PROVINCE OF ONTARIO

March 10, 2017

KOTT

RECEIVED
TOWN OF MILTON
MAR 20, 2017
17-1078
BUILDING DIVISION



TOTAL WEIGHT = 126 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 3	2x4	DRY	No.2	SPF
3 - 4	2x4	DRY	No.2	SPF
4 - 6	2x4	DRY	No.2	SPF
6 - 7	2x4	DRY	No.2	SPF
7 - 9	2x4	DRY	No.2	SPF
17 - 2	2x4	DRY	No.2	SPF
10 - 9	2x4	DRY	No.2	SPF
17 - 13	2x4	DRY	No.2	SPF
13 - 10	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-t	MT20	4.0	6.0	1.75	2.75
3	TTWW-m	MT20	5.0	8.0	2.00	3.25
4	TTWW-m	MT20	4.0	6.0	2.25	3.00
5	TMWW+t	MT20	3.0	4.0	1.75	0.75
6	TTW+p	MT20	3.0	6.0		
7	TS-t	MT20	3.0	6.0		
8	TMWW-t	MT20	3.0	4.0	1.50	1.75
9	TMVW-t	MT20	4.0	6.0	1.75	2.75
10	BMV1+p	MT20	2.0	4.0		
11	BMWW-t	MT20	3.0	10.0	1.50	3.75
12	BMWWW-t	MT20	4.0	6.0	1.75	3.00
13	BS-t	MT20	3.0	6.0		
14	BMWW-t	MT20	3.0	5.0		
15	BMWW-t	MT20	4.0	4.0	1.75	1.50
16	BMWW-t	MT20	4.0	5.0	1.75	1.50
17	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
17	1583	0	1618	221	-730	IN-SX 5-8	IN-SX 5-8
10	1478	0	1510	0	-635	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

PROVIDE ANCHORAGE AT BEARING JOINT 17 FOR 730 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 10 FOR 635 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 221 LBS FACTORED HORIZONTAL REACTION AT JOINT 17

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
17	1109	789 / 0	0 / 0	0 / 0	88 / -727	320 / 0	0 / 0
10	1037	726 / 0	0 / 0	0 / 0	79 / -654	312 / 0	0 / 0
HORIZONTAL REACTIONS							
17	---	0 / 0	0 / 0	0 / 0	158 / -132	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 17

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 5.82 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 8-12. DBS = 20-0-0 . CBF = 80 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
1- 2	0 / 23	-77.3	-77.3	0.10 (1)		10.00
2- 3	-2118 / 932	-77.3	-77.3	0.38 (7)		4.48
3- 4	-2631 / 1203	-77.3	-77.3	0.23 (7)		4.22
4- 5	-2270 / 1027	-77.3	-77.3	0.40 (7)		4.33
5- 6	-1664 / 811	-77.3	-77.3	0.37 (7)		4.93
6- 7	-1681 / 817	-77.3	-77.3	0.70 (8)		4.47
7- 8	-1681 / 817	-77.3	-77.3	0.70 (8)		4.47
8- 9	-2240 / 938	-77.3	-77.3	0.73 (8)		3.89
17- 2	-1581 / 754	0.0	0.0	0.16 (1)		6.61
10- 9	-1457 / 669	0.0	0.0	0.14 (1)		6.82
17-16	-204 / 187	-17.5	-17.5	0.08 (11)		6.25
16-15	-900 / 1919	-17.5	-17.5	0.32 (1)		6.25
15-14	-1211 / 2691	-17.5	-17.5	0.48 (1)		5.82
14-13	-85 / 20	-17	-5	0		6.25
13-12	/ 20	-17	-5	0.4	1)	6.25
12-11	/ 20	-17	-5	0.4	1)	6.25
11-10	/ 1	-17	-5	0.2	11)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI
16- 3	-265 / 178	0.05 (1)
3-15	-473 / 1181	0.26 (1)
15- 4	-871 / 416	0.17 (1)
4-14	-737 / 432	0.41 (3)
14- 5	-179 / 491	0.11 (1)
5-12	-904 / 602	0.93 (3)
12- 6	-520 / 1123	0.68 (7)
12- 8	-688 / 550	0.31 (4)
11- 8	-76 / 186	0.04 (8)
2-16	-713 / 1914	0.42 (1)
11- 9	-732 / 2029	0.73 (8)

WIND
{40-0
COE
WIN
{OPE
FRO
PRESSURE OF { 9.0} PSF AT
EXTERIOR ZONE
SING SYSTEM}.INTERNAL
MAY BE LOCATED ON
LEAST {0-0} FT-IN-SX AWAY

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.04")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (1.04")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.25")

CSI: TC=0.73 (8-9:8) , BC=0.48 (14-15:1) , WB=0.93 (5-12:3) , SSI=0.24 (8-9:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

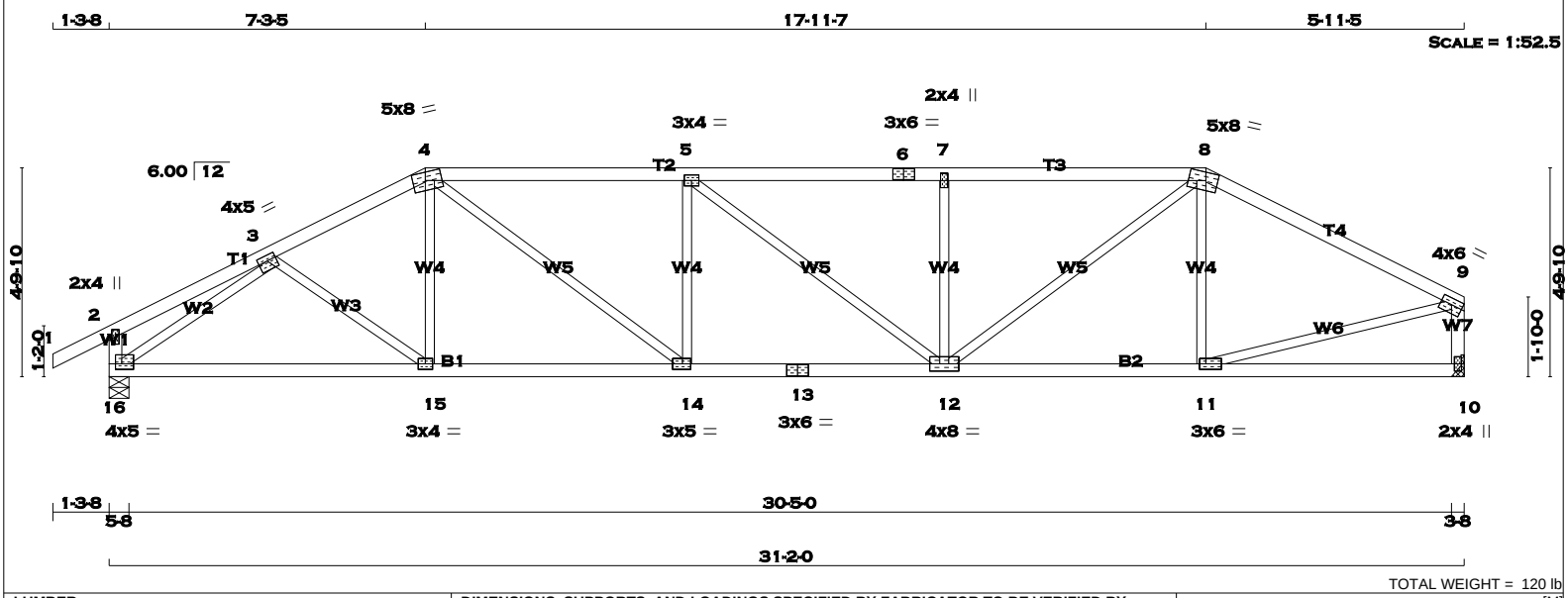
JSI GRIP= 0.89 (9) (INPUT = 0.90)
JSI METAL= 0.59 (9) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
T.L. WISE
100083566
PROVINCE OF ONTARIO

March 10, 2017

KOTT

RECEIVED
TOWN OF MILTON
MAR 23, 2017
17-4978
BUILDING DIVISION



TOTAL WEIGHT = 120 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 6	2x4	DRY	No.2	SPF
6 - 8	2x4	DRY	No.2	SPF
8 - 9	2x4	DRY	No.2	SPF
16 - 2	2x4	DRY	No.2	SPF
10 - 9	2x4	DRY	No.2	SPF
16 - 13	2x4	DRY	No.2	SPF
13 - 10	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

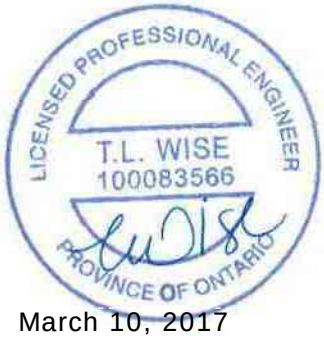
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMV+p	MT20	2.0	4.0		
3	TMWW-t	MT20	4.0	5.0	1.50	2.50
4	TTWW-m	MT20	5.0	8.0	2.25	3.50
5	TMWW-t	MT20	3.0	4.0		
6	TS-t	MT20	3.0	6.0		
7	TMW+w	MT20	2.0	4.0		
8	TTWW-m	MT20	5.0	8.0	2.25	3.25
9	TMVW-t	MT20	4.0	6.0	1.75	Edge
10	BMV1+p	MT20	2.0	4.0		
11	BMWW-t	MT20	3.0	6.0	1.50	1.75
12	BMWWW-t	MT20	4.0	8.0		
13	BS-t	MT20	3.0	6.0		
14	BMWW-t	MT20	3.0	5.0	1.50	2.25
15	BMWW-t	MT20	3.0	4.0		
16	BMVW1-t	MT20	4.0	5.0	1.50	1.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.



March 10, 2017

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
16	1583	0	1637	174	-819	5-8	5-8
10	1478	0	1513	0	-750	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

PROVIDE ANCHORAGE AT BEARING JOINT 16 FOR 819 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 10 FOR 750 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 174 LBS FACTORED HORIZONTAL REACTION AT JOINT 16

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
16	1109	789 / 0	0 / 0	0 / 0	136 / -791	320 / 0	0 / 0
10	1037	726 / 0	0 / 0	0 / 0	87 / -736	312 / 0	0 / 0
HORIZONTAL REACTIONS							
16	---	0 / 0	0 / 0	0 / 0	124 / -83	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 16

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.81 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 5.71 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 23	-77.3	-77.3	0.10 (1)	10.00	3-15	0 / 148	0.03 (7)
2- 3	-9 / 114	-77.3	-77.3	0.15 (7)	10.00	15- 4	0 / 105	0.04 (11)
3- 4	-2165 / 1140	-77.3	-77.3	0.29 (7)	4.53	4-14	-558 / 921	0.60 (8)
4- 5	-2666 / 1444	-77.3	-77.3	0.55 (1)	3.81	14- 5	-457 / 402	0.15 (1)
5- 6	-2574 / 1380	-77.3	-77.3	0.54 (1)	3.87	5-12	-117 / 98	0.12 (3)
6- 7	-2574 / 1380	-77.3	-77.3	0.54 (1)	3.87	12- 7	-527 / 407	0.17 (1)
7- 8	-2574 / 1379	-77.3	-77.3	0.54 (1)	3.88	12- 8	-652 / 1164	0.70 (7)
8- 9	-1847 / 931	-77.3	-77.3	0.61 (1)	4.30	11- 8	-298 / 261	0.10 (1)
16- 2	-226 / 204	0.0	0.0	0.03 (7)	7.81	16- 3	-2292 / 1101	0.74 (1)
10- 9	-1470 / 780	0.0	0.0	0.16 (1)	6.80	11- 9	-723 / 1706	0.46 (8)
16-15	-1003 / 1873	-17.5	-17.5	0.40 (1)	6.16			
15-14	-883 / 1935	-17.5	-17.5	0.41 (1)	6.25			
14-13	-1259 / 2672	-17.5	-17.5	0.47 (1)	5.71			
13-12	-1259 / 2672	-17.5	-17.5	0.47 (1)	5.71			
12-11	-665 / 1647	-17.5	-17.5	0.34 (1)	6.25			
11-10	-15 / 32	-17.5	-17.5	0.15 (11)	6.25			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN AND TERRAIN WITH SCATTERED OBSTRUCTIONS} BUILDING TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVES.

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

RECEIVED
TOWN OF MILTON
MAR 29, 2017
17-4978
BUILDING DIVISION

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.04")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (1.04")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.27")

CSI: TC=0.61 (8-9:1) , BC=0.47 (12-14:1) , WB=0.74 (3-16:1) , SSI=0.22 (4-5:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

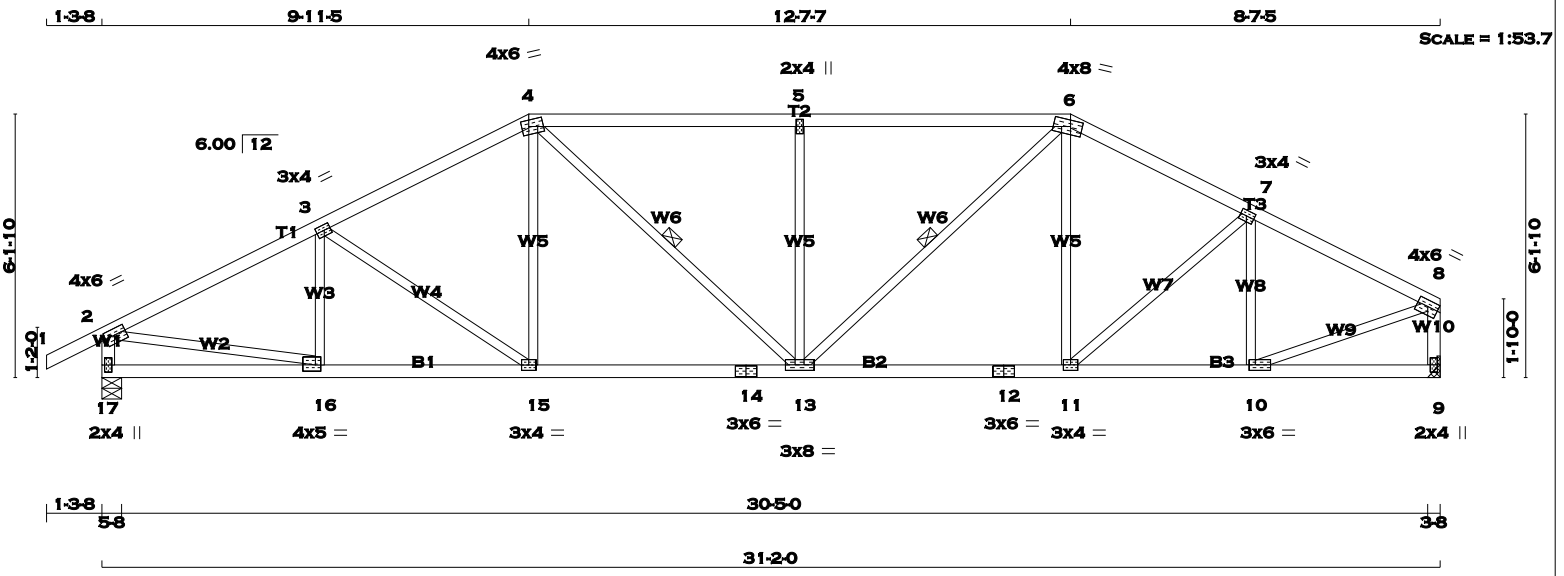
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (14) (INPUT = 0.90)
JSI METAL= 0.76 (13) (INPUT = 1.00)



TOTAL WEIGHT = 126 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 6	2x4	DRY	No.2	SPF
6 - 8	2x4	DRY	No.2	SPF
17 - 2	2x4	DRY	No.2	SPF
9 - 8	2x4	DRY	No.2	SPF
17 - 14	2x4	DRY	No.2	SPF
14 - 12	2x4	DRY	No.2	SPF
12 - 9	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-t	MT20	4.0	6.0	1.75	2.75
3	TMWW-t	MT20	3.0	4.0	1.50	1.75
4	TTWW-m	MT20	4.0	6.0	1.75	2.00
5	TMW+w	MT20	2.0	4.0		
6	TTWW-m	MT20	4.0	8.0	1.75	3.25
7	TMWW-t	MT20	3.0	4.0	1.50	1.75
8	TMVW-t	MT20	4.0	6.0	1.75	Edge
9	BMV1+p	MT20	2.0	4.0		
10	BMWW-t	MT20	3.0	6.0	1.50	1.75
11	BMWW-t	MT20	3.0	4.0		
12	BS-t	MT20	3.0	6.0		
13	BMWWW-t	MT20	3.0	8.0		
14	BS-t	MT20	3.0	6.0		
15	BMWW-t	MT20	3.0	4.0		
16	BMWW-t	MT20	4.0	5.0	1.75	1.50
17	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
17	1583	0	1633	200	-797	5-8	5-8
9	1478	0	1503	0	-725	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

PROVIDE ANCHORAGE AT BEARING JOINT 17 FOR 797 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 9 FOR 725 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 200 LBS FACTORED HORIZONTAL REACTION AT JOINT 17

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
17	1109	789 / 0	0 / 0	0 / 0	124 / -775	320 / 0	0 / 0
9	1037	726 / 0	0 / 0	0 / 0	62 / -718	312 / 0	0 / 0
HORIZONTAL REACTIONS							
17	---	0 / 0	0 / 0	0 / 0	143 / -101	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 17

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.27 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.21 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 4-13, 6-13. DBS = 20-0-0 . CBF = 45 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 23	-77.3	-77.3	0.10 (1)	10.00	16- 3	-249 / 221	0.05 (1)
2- 3	-2204 / 1094	-77.3	-77.3	0.43 (7)	4.40	3-15	-263 / 278	0.16 (3)
3- 4	-2016 / 1070	-77.3	-77.3	0.43 (7)	4.58	15- 4	-94 / 252	0.05 (1)
4- 5	-2110 / 1153	-77.3	-77.3	0.52 (7)	4.27	4-13	-326 / 437	0.15 (8)
5- 6	-2110 / 1153	-77.3	-77.3	0.52 (7)	4.27	13- 5	-631 / 488	0.36 (3)
6- 7	-1839 / 990	-77.3	-77.3	0.35 (8)	4.81	13- 6	-412 / 657	0.20 (7)
7- 8	-1749 / 886	-77.3	-77.3	0.33 (8)	4.90	11- 6	-26 / 96	0.03 (11)
17- 2	-1592 / 820	0.0	0.0	0.16 (1)	6.60	11- 7	-29 / 138	0.03 (5)
9- 8	-1467 / 746	0.0	0.0	0.16 (1)	6.81	10- 7	-475 / 306	0.12 (1)
						2-16	-871 / 2007	0.44 (1)
17-16	-182 / 133	-17.5	-17.5	0.09 (11)	6.25	10- 8	-745 / 1673	0.37 (1)
16-15	-1009 / 2012	-17.5	-17.5	0.37 (1)	6.21			
15-14	-782 / 1800	-17.5	-17.5	0.34 (1)	6.25			
14-13	-78 / 15	-17.5	-17.5	0.03 (1)	6.25			
13-12	/ 10	-17.5	-17.5	0.3 (1)	6.25			
12-11	/ 10	-17.5	-17.5	0.3 (1)	6.25			
11-10	-61 / 15	-17.5	-17.5	0.3 (1)	6.25			
10- 9	-15 / 32	-17.5	-17.5	0.0 (11)	6.25			

WIND {40-0 COE WIN {OPE FRO PRESSURE (20) PSF AT EXTERNAL PEAK STING SYSTEM; INTERNAL MAY BE LOCATED ON LEAST {0-0 FROM SIX AWAY

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.04")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (1.04")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.52 (4-5:7) , BC=0.37 (15-16:1) , WB=0.44 (2-16:1) , SSI=0.24 (4-5:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (10) (INPUT = 0.90)
JSI METAL= 0.59 (16) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
T.L. WISE
100083566
PROVINCE OF ONTARIO
March 10, 2017

KOTT

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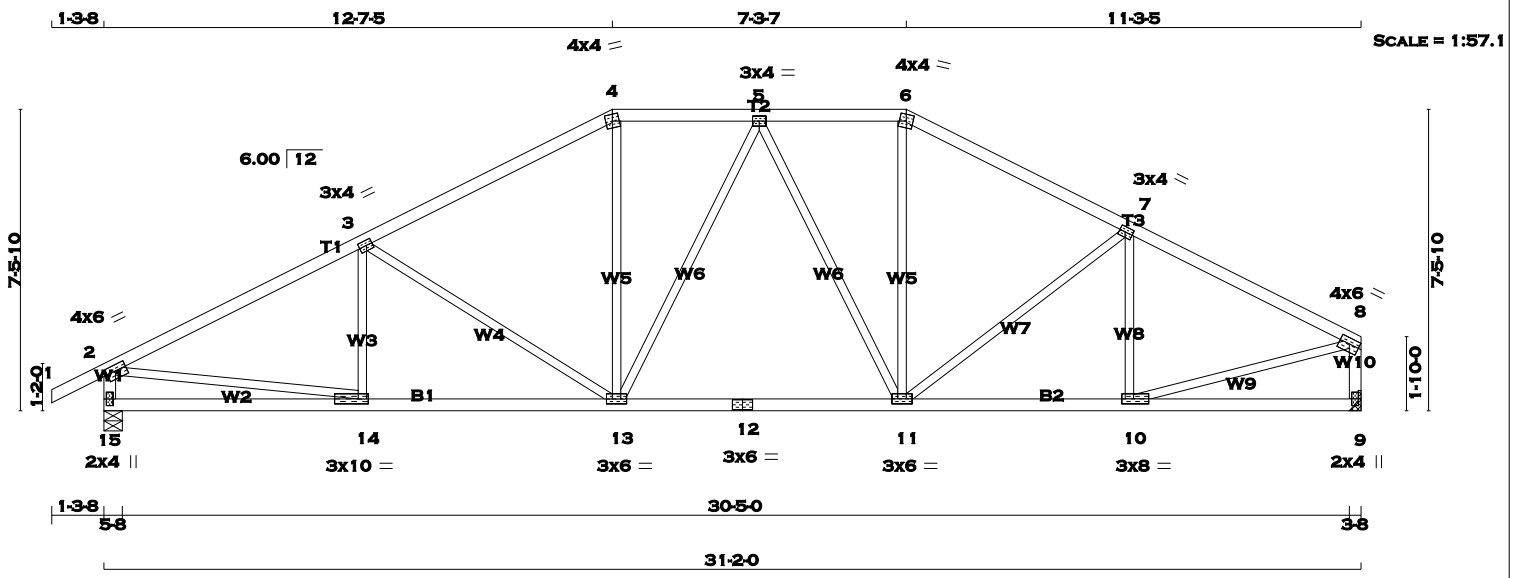

TOTAL WEIGHT = 130 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 6	2x4	DRY	No.2	SPF
6 - 8	2x4	DRY	No.2	SPF
15 - 2	2x4	DRY	No.2	SPF
9 - 8	2x4	DRY	No.2	SPF
15 - 12	2x4	DRY	No.2	SPF
12 - 9	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-t	MT20	4.0	6.0	1.75	2.75
3	TMWW-t	MT20	3.0	4.0	1.50	1.75
4	TTW-m	MT20	4.0	4.0		
5	TMWW-t	MT20	3.0	4.0		
6	TTW-m	MT20	4.0	4.0		
7	TMWW-t	MT20	3.0	4.0	1.50	1.75
8	TMVW-t	MT20	4.0	6.0	1.75	Edge
9	BMV1+p	MT20	2.0	4.0		
10	BMWW-t	MT20	3.0	8.0	1.50	3.50
11	BMWWW-t	MT20	3.0	6.0		
12	BS-t	MT20	3.0	6.0		
13	BMWWW-t	MT20	3.0	6.0		
14	BMWW-t	MT20	3.0	10.0	1.50	3.00
15	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
15	1583	0	1626	226	-768	5-8	5-8
9	1478	0	1500	0	-693	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

PROVIDE ANCHORAGE AT BEARING JOINT 15 FOR 768 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 9 FOR 693 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 226 LBS FACTORED HORIZONTAL REACTION AT JOINT 15

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
15	1109	789 / 0	0 / 0	0 / 0	108 / -754	320 / 0	0 / 0
9	1037	726 / 0	0 / 0	0 / 0	56 / -695	312 / 0	0 / 0
HORIZONTAL REACTIONS							
15	---	0 / 0	0 / 0	0 / 0	161 / -120	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 15

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.15 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.23 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
1- 2	0 / 23	-77.3	-77.3	0.10 (1)	10.00
2- 3	-2227 / 1059	-77.3	-77.3	0.60 (7)	4.15
3- 4	-1816 / 939	-77.3	-77.3	0.59 (7)	4.55
4- 5	-1612 / 923	-77.3	-77.3	0.24 (7)	5.14
5- 6	-1530 / 880	-77.3	-77.3	0.24 (8)	5.24
6- 7	-1729 / 900	-77.3	-77.3	0.49 (8)	4.75
7- 8	-1850 / 894	-77.3	-77.3	0.49 (8)	4.60
15- 2	-1578 / 798	0.0	0.0	0.16 (1)	6.62
9- 8	-1457 / 721	0.0	0.0	0.16 (1)	6.82
15-14	-208 / 159	-17.5	-17.5	0.15 (11)	6.25
14-13	-995 / 2045	-17.5	-17.5	0.40 (1)	6.23
13-12	-641 / 1668	-17.5	-17.5	0.35 (1)	6.25
12-11	-641 / 1668	-17.5	-17.5	0.35 (1)	6.25
11-10	-663 / 1678	-17.5	-17.5	0.35 (1)	6.25
10- 9	-15 / 32	-17.5	-17.5	0.12 (11)	6.25

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
14- 3	-152 / 201	0.04 (1)
3-13	-512 / 442	0.57 (3)
13- 4	-214 / 492	0.21 (8)
13- 5	-131 / 214	0.17 (4)
5-11	-290 / 275	0.39 (3)
11- 6	-220 / 487	0.22 (7)
11- 7	-238 / 292	0.23 (4)
10- 7	-342 / 265	0.11 (1)
2-14	-817 / 2022	0.58 (7)
10- 8	-717 / 1735	0.41 (8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FRO[illegible]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.04")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.04")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.60 (2-3:7) , BC=0.40 (13-14:1) , WB=0.58 (2-14:7) , SSI=0.21 (2-3:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (14) (INPUT = 0.90)
JSI METAL= 0.60 (2) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
T.L. WISE
100083566
PROVINCE OF ONTARIO

March 10, 2017

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

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MAR 29, 2017
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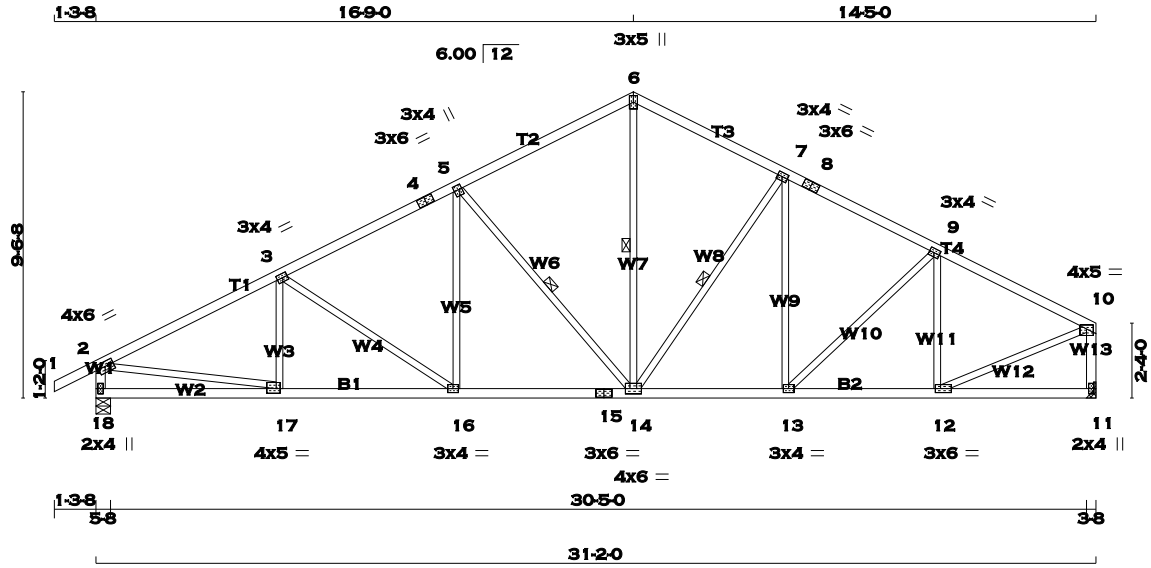


TOTAL WEIGHT = 136 lb

[M][F]

LUMBER

N. L. G. A. RULES

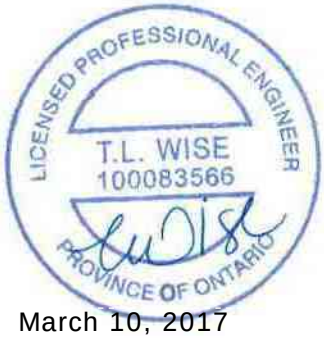
CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 6	2x4	DRY	No.2	SPF
6 - 8	2x4	DRY	No.2	SPF
8 - 10	2x4	DRY	No.2	SPF
18 - 2	2x4	DRY	No.2	SPF
11 - 10	2x4	DRY	No.2	SPF
18 - 15	2x4	DRY	No.2	SPF
15 - 11	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-t	MT20	4.0	6.0	1.75	2.75
3, 7, 9						
3	TMWW-t	MT20	3.0	4.0	1.50	1.75
4	TS-t	MT20	3.0	6.0		
5	TMWW+t	MT20	3.0	4.0	1.75	0.75
6	TTW+p	MT20	3.0	5.0		
8	TS-t	MT20	3.0	6.0		
10	TMVW-p	MT20	4.0	5.0	1.50	2.50
11	BMV1+p	MT20	2.0	4.0		
12	BMWW-t	MT20	3.0	6.0	1.50	2.00
13	BMWW-t	MT20	3.0	4.0		
14	BMWWW-t	MT20	4.0	6.0		
15	BS-t	MT20	3.0	6.0		
16	BMWW-t	MT20	3.0	4.0		
17	BMWW-t	MT20	4.0	5.0	1.75	1.50
18	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.



March 10, 2017

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
18	1583	0	1616	288	-719	5-8	5-8
11	1478	0	1507	0	-628	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

PROVIDE ANCHORAGE AT BEARING JOINT 18 FOR 719 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 11 FOR 628 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 288 LBS FACTORED HORIZONTAL REACTION AT JOINT 18

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
18	1109	789 / 0	0 / 0	0 / 0	83 / -719	320 / 0	0 / 0
11	1037	726 / 0	0 / 0	0 / 0	73 / -649	312 / 0	0 / 0
HORIZONTAL REACTIONS							
18	---	0 / 0	0 / 0	0 / 0	206 / -148	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 18

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.32 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 5-14, 6-14, 7-14. DBS = 20-0-0 . CBF = 84 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 23	-77.3	-77.3	0.10 (1)	10.00	17- 3	-189 / 192	0.05 (1)
2- 3	-2188 / 960	-77.3	-77.3	0.43 (7)	4.32	3-16	-326 / 309	0.26 (3)
3- 4	-1918 / 907	-77.3	-77.3	0.42 (7)	4.63	16- 5	-106 / 277	0.08 (7)
4- 5	-1918 / 907	-77.3	-77.3	0.42 (7)	4.63	5-14	-713 / 539	0.34 (3)
5- 6	-1437 / 755	-77.3	-77.3	0.38 (7)	5.18	14- 6	-475 / 940	0.28 (7)
6- 7	-1436 / 784	-77.3	-77.3	0.30 (8)	5.27	14- 7	-421 / 404	0.19 (4)
7- 8	-1669 / 796	-77.3	-77.3	0.33 (8)	4.97	13- 7	-37 / 80	0.03 (8)
8- 9	-1669 / 796	-77.3	-77.3	0.33 (8)	4.97	13- 9	-25 / 135	0.03 (8)
9-10	-1601 / 703	-77.3	-77.3	0.32 (8)	5.04	12- 9	-490 / 284	0.17 (1)
18- 2	-1572 / 746	0.0	0.0	0.16 (1)	6.63	2-17	-734 / 1984	0.44 (1)
11-10	-1471 / 652	0.0	0.0	0.18 (1)	6.79	12-10	-562 / 1543	0.34 (1)
18-17	-270 / 198	-17.5	-17.5	0.12 (11)	6.25			
17-16	-967 / 2014	-17.5	-17.5	0.36 (1)	6.25			
16-15	-711 / 1744	-17.5	-17.5	0.33 (1)	6.25			
15-14	-711 / 1744	-17.5	-17.5	0.33 (1)	6.25			
14-13	-451 / 1490	-17.5	-17.5	0.29 (1)	6.25			
13-12	-480 / 1429	-17.5	-17.5	0.27 (1)	6.25			
12-11	-20 / 42	-17.5	-17.5	0.10 (11)	6.25			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.04")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.04")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.43 (2-3:7) , BC=0.36 (16-17:1) , WB=0.44 (2-17:1) , SSI=0.18 (2-3:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

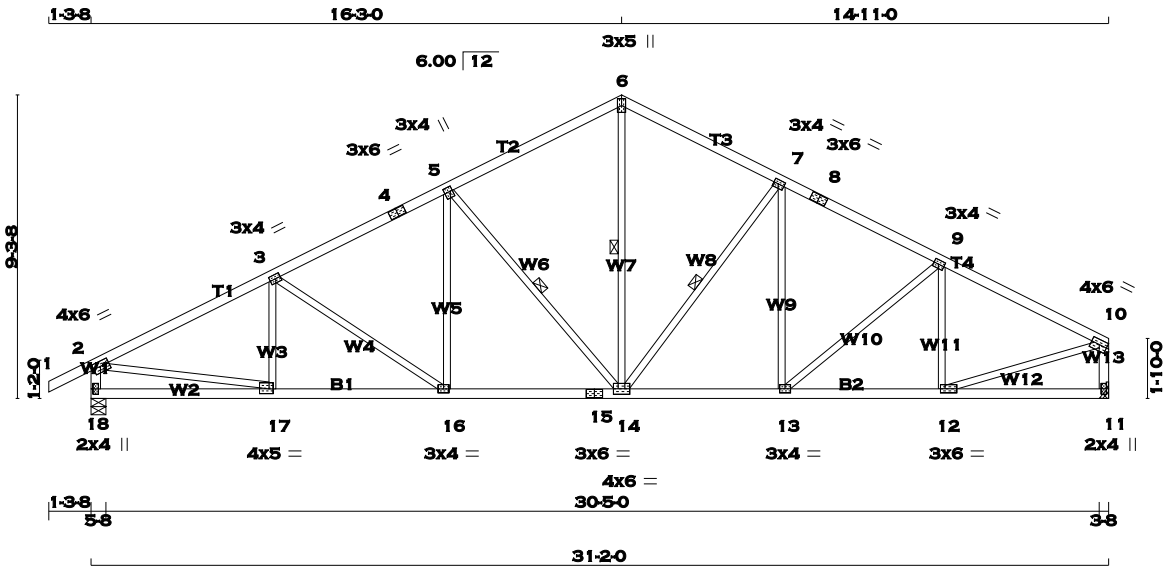
JSI GRIP= 0.90 (10) (INPUT = 0.90)
JSI METAL= 0.59 (17) (INPUT = 1.00)

CONTINUED ON PAGE 2

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



TOTAL WEIGHT = 4 X 134 = 534 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 6	2x4	DRY	No.2	SPF
6 - 8	2x4	DRY	No.2	SPF
8 - 10	2x4	DRY	No.2	SPF
18 - 2	2x4	DRY	No.2	SPF
11 - 10	2x4	DRY	No.2	SPF
18 - 15	2x4	DRY	No.2	SPF
15 - 11	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

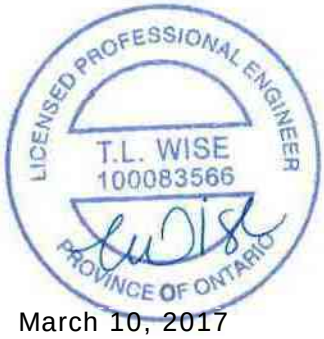
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-t	MT20	4.0	6.0	1.75	2.75
3, 7, 9						
3	TMWW-t	MT20	3.0	4.0	1.50	1.75
4	TS-t	MT20	3.0	6.0		
5	TMWW+t	MT20	3.0	4.0	1.75	0.75
6	TTW+p	MT20	3.0	5.0		
8	TS-t	MT20	3.0	6.0		
10	TMVW-t	MT20	4.0	6.0	1.75	Edge
11	BMV1+p	MT20	2.0	4.0		
12	BMWW-t	MT20	3.0	6.0	1.50	1.75
13	BMWW-t	MT20	3.0	4.0		
14	BMWWW-t	MT20	4.0	6.0		
15	BS-t	MT20	3.0	6.0		
16	BMWW-t	MT20	3.0	4.0		
17	BMWW-t	MT20	4.0	5.0	1.75	1.50
18	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.



March 10, 2017

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
18	1583	0	1615	261	-715	5-8	5-8
11	1478	0	1510	0	-636	HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-8	

PROVIDE ANCHORAGE AT BEARING JOINT 18 FOR 715 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 11 FOR 636 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 261 LBS FACTORED HORIZONTAL REACTION AT JOINT 18

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
18	1109	789 / 0	0 / 0	0 / 0	80 / -716	320 / 0	0 / 0
11	1037	726 / 0	0 / 0	0 / 0	79 / -655	312 / 0	0 / 0
HORIZONTAL REACTIONS							
18	---	0 / 0	0 / 0	0 / 0	186 / -145	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 18

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.34 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 5-14, 6-14, 7-14. DBS = 20-0-0 . CBF = 80 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 23	-77.3	-77.3	0.10 (1)	10.00	17- 3	-201 / 192	0.05 (1)
2- 3	-2182 / 952	-77.3	-77.3	0.41 (7)	4.34	3-16	-298 / 293	0.22 (3)
3- 4	-1938 / 908	-77.3	-77.3	0.40 (7)	4.63	16- 5	-100 / 259	0.07 (7)
4- 5	-1938 / 908	-77.3	-77.3	0.40 (7)	4.63	5-14	-682 / 520	0.30 (3)
5- 6	-1480 / 771	-77.3	-77.3	0.37 (7)	5.13	14- 6	-483 / 980	0.26 (7)
6- 7	-1480 / 789	-77.3	-77.3	0.32 (8)	5.19	14- 7	-510 / 439	0.22 (4)
7- 8	-1788 / 839	-77.3	-77.3	0.35 (8)	4.82	13- 7	-58 / 121	0.05 (8)
8- 9	-1788 / 839	-77.3	-77.3	0.35 (8)	4.82	13- 9	-65 / 182	0.05 (4)
9-10	-1814 / 787	-77.3	-77.3	0.34 (8)	4.76	12- 9	-387 / 253	0.11 (1)
18- 2	-1573 / 741	0.0	0.0	0.16 (1)	6.63	2-17	-730 / 1980	0.44 (1)
11-10	-1471 / 661	0.0	0.0	0.16 (1)	6.79	12-10	-629 / 1690	0.38 (1)
18-17	-243 / 194	-17.5	-17.5	0.12 (11)	6.25			
17-16	-954 / 2008	-17.5	-17.5	0.36 (1)	6.25			
16-15	-712 / 1762	-17.5	-17.5	0.33 (1)	6.25			
15-14	-712 / 1762	-17.5	-17.5	0.33 (1)	6.25			
14-13	-489 / 1594	-17.5	-17.5	0.31 (1)	6.25			
13-12	-573 / 1616	-17.5	-17.5	0.30 (1)	6.25			
12-11	-15 / 32	-17.5	-17.5	0.10 (11)	6.25			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.04")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.04")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.41 (2-3:7) , BC=0.36 (16-17:1) , WB=0.44 (2-17:1) , SSI=0.18 (2-3:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

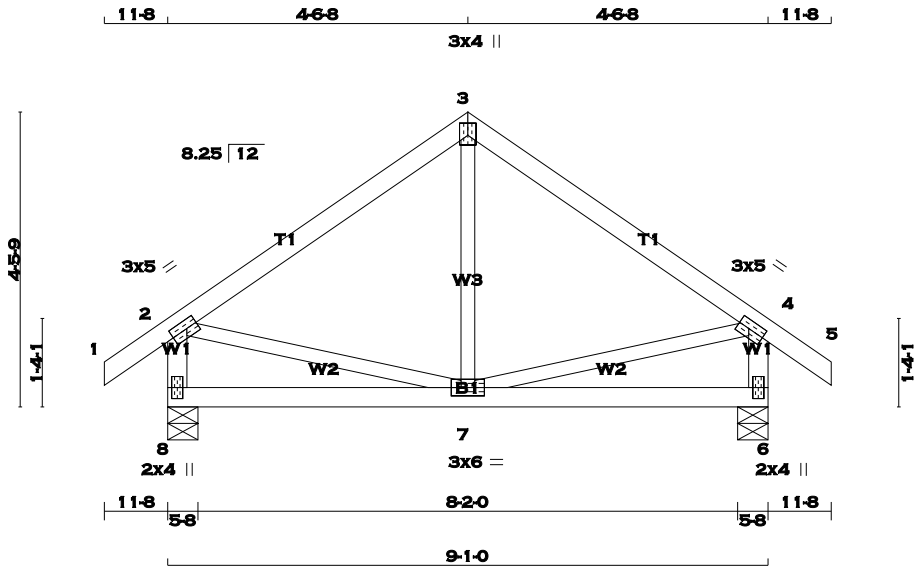
JSI GRIP= 0.90 (12) (INPUT = 0.90)
JSI METAL= 0.59 (17) (INPUT = 1.00)

CONTINUED ON PAGE 2

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



TOTAL WEIGHT = 6 X 38 = 229 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 3	2x4	DRY	No.2	SPF
3 - 5	2x4	DRY	No.2	SPF
8 - 2	2x4	DRY	No.2	SPF
6 - 4	2x4	DRY	No.2	SPF
8 - 6	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-t	MT20	3.0	5.0	1.50	2.00
3	TTW+p	MT20	3.0	4.0	2.25	1.50
4	TMVW-t	MT20	3.0	5.0	1.50	2.00
6	BMV1+p	MT20	2.0	4.0		
7	BMWWW-t	MT20	3.0	6.0		
8	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
8	511	0	538	-209	-221	5-8	5-8
6	511	0	538	0	-221	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 8 FOR 221 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 6 FOR 221 LBS FACTORED UPLIFT

PROVIDE FOR 209 LBS FACTORED HORIZONTAL REACTION AT JOINT 8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
8	357	260 / 0	0 / 0	0 / 0	67 / -220	97 / 0	0 / 0
6	357	260 / 0	0 / 0	0 / 0	67 / -220	97 / 0	0 / 0
HORIZONTAL REACTIONS							
8	---	0 / 0	0 / 0	0 / 0	149 / -149	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 8, 6

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 23	-77.3	-77.3	0.06 (1)	10.00	7- 3	-15 / 74	0.03 (11)
2- 3	-327 / 167	-77.3	-77.3	0.31 (7)	6.25	2- 7	-60 / 283	0.06 (1)
3- 4	-327 / 167	-77.3	-77.3	0.31 (8)	6.25	7- 4	-60 / 283	0.06 (1)
4- 5	0 / 23	-77.3	-77.3	0.06 (1)	10.00			
8- 2	-507 / 242	0.0	0.0	0.05 (1)	7.81			
6- 4	-507 / 242	0.0	0.0	0.05 (1)	7.81			
8- 7	-189 / 200	-17.5	-17.5	0.10 (11)	6.25			
7- 6	-9 / 20	-17.5	-17.5	0.10 (11)	10.00			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.30")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.30")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.31 (2-3:7) , BC=0.10 (6-7:11) , WB=0.06 (2-7:1) , SSI=0.12 (2-3:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.47 (7) (INPUT = 0.90)
JSI METAL= 0.14 (2) (INPUT = 1.00)

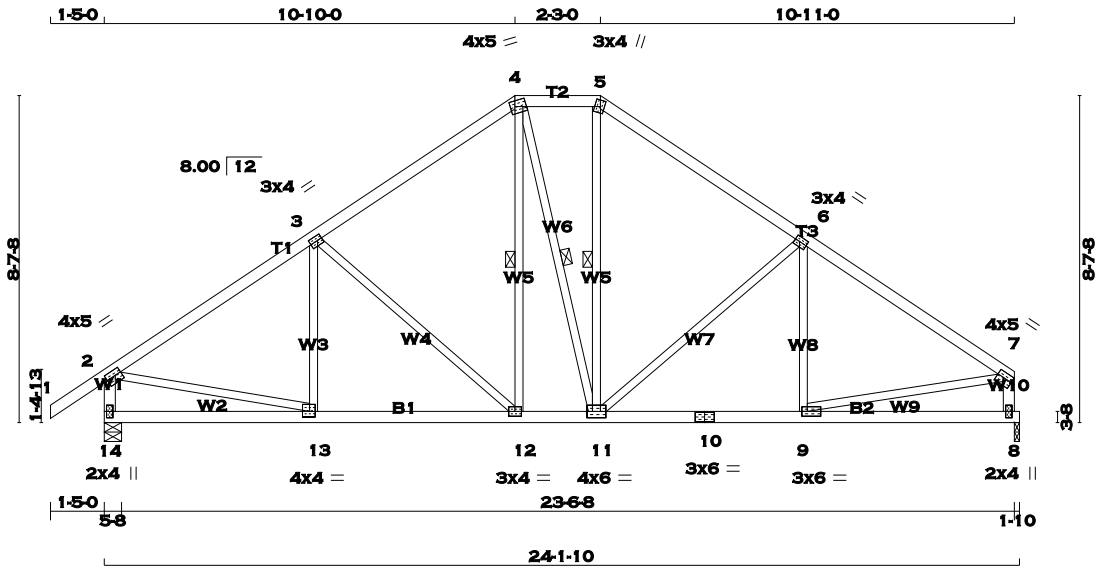
LICENSED PROFESSIONAL ENGINEER
T.L. WISE
100083566
PROVINCE OF ONTARIO

March 10, 2017

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

RECEIVED
TOWN OF MILTON
MAR 29, 2017
17-4978
BUILDING DIVISION



SCALE = 1:60.8

TOTAL WEIGHT = 111 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 5	2x4	DRY	No.2	SPF
5 - 7	2x4	DRY	No.2	SPF
14 - 2	2x4	DRY	No.2	SPF
8 - 7	2x4	DRY	No.2	SPF
14 - 10	2x4	DRY	No.2	SPF
10 - 8	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-t	MT20	4.0	5.0	1.75	2.00
3	TMWW-t	MT20	3.0	4.0	1.50	1.50
4	TTWW-m	MT20	4.0	5.0	1.75	1.50
5	TTW+m	MT20	3.0	4.0	2.00	1.25
6	TMWW-t	MT20	3.0	4.0	1.50	1.50
7	TMVW-t	MT20	4.0	5.0	1.75	Edge
8	BMV1+p	MT20	2.0	4.0		
9	BMWW-t	MT20	3.0	6.0	1.50	1.75
10	BS-t	MT20	3.0	6.0		
11	BMWWW-t	MT20	4.0	6.0		
12	BMWW-t	MT20	3.0	4.0		
13	BMWW-t	MT20	4.0	4.0	1.75	1.75
14	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
14	1254	0	1334	371	-566	5-8	5-8
8	1138	0	1206	0	-500	1-10	1-10

PROVIDE ANCHORAGE AT BEARING JOINT 14 FOR 566 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 8 FOR 500 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 371 LBS FACTORED HORIZONTAL REACTION AT JOINT 14

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
14	877	629 / 0	0 / 0	0 / 0	199 / -564	249 / 0	0 / 0
8	799	559 / 0	0 / 0	0 / 0	170 / -512	240 / 0	0 / 0
HORIZONTAL REACTIONS							
14	---	0 / 0	0 / 0	0 / 0	265 / -257	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 14, 8

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.23 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 4-12, 4-11, 5-11. DBS = 20-0-0 . CBF = 25 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
1- 2	0 / 32	-77.3	-77.3	0.12 (1)		10.00
2- 3	-1355 / 592	-77.3	-77.3	0.48 (7)		5.25
3- 4	-1069 / 591	-77.3	-77.3	0.48 (7)		5.74
4- 5	-876 / 576	-77.3	-77.3	0.13 (8)		6.25
5- 6	-1077 / 593	-77.3	-77.3	0.48 (8)		5.76
6- 7	-1360 / 588	-77.3	-77.3	0.49 (8)		5.23
14- 2	-1294 / 592	0.0	0.0	0.13 (1)		7.14
8- 7	-1166 / 527	0.0	0.0	0.11 (1)		7.42
14-13	-350 / 349	-17.5	-17.5	0.13 (11)		6.25
13-12	-542 / 1217	-17.5	-17.5	0.24 (1)		6.25
12-11	-232 / 888	-17.5	-17.5	0.18 (1)		6.25
11-10	-351 / 1146	-17.5	-17.5	0.24 (1)		6.25
10- 9	-351 / 1146	-17.5	-17.5	0.24 (1)		6.25
9- 8	[illegible]			[illegible]		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI
13- 3	-112 / 150	0.04 (4)
3-12	-443 / 416	0.44 (3)
12- 4	-227 / 345	0.10 (7)
4-11	-155 / 155	0.08 (5)
11- 5	-188 / 338	0.09 (7)
11- 6	-455 / 415	0.45 (4)
9- 6	-104 / 153	0.04 (3)
2-13	-349 / 1154	0.25 (1)
9- 7	-378 / 1165	0.25 (1)

WIND LOAD APPLIED FROM REFERENCE VELOCITY PRESSURE OF [illegible] PSF AT {40-0-0 X IN-SX} REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM/ENTIRE WIN[illegible] MAY BE LOCATED ON {OPE[illegible] LEAST {1-0-0 X IN-SX} AWAY FRO[illegible]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF	
	DL =	3.0	PSF	
BOT CH.	LL =	0.0	PSF	
	DL =	7.0	PSF	
TOTAL LOAD	=	33.3	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.80")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.80")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.49 (6-7:8) , BC=0.24 (12-13:1) , WB=0.45 (6-11:4) , SSI=0.18 (6-7:8)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (9) (INPUT = 0.90)
JSI METAL= 0.46 (2) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
T.L. WISE
100083566
PROVINCE OF ONTARIO

March 10, 2017

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

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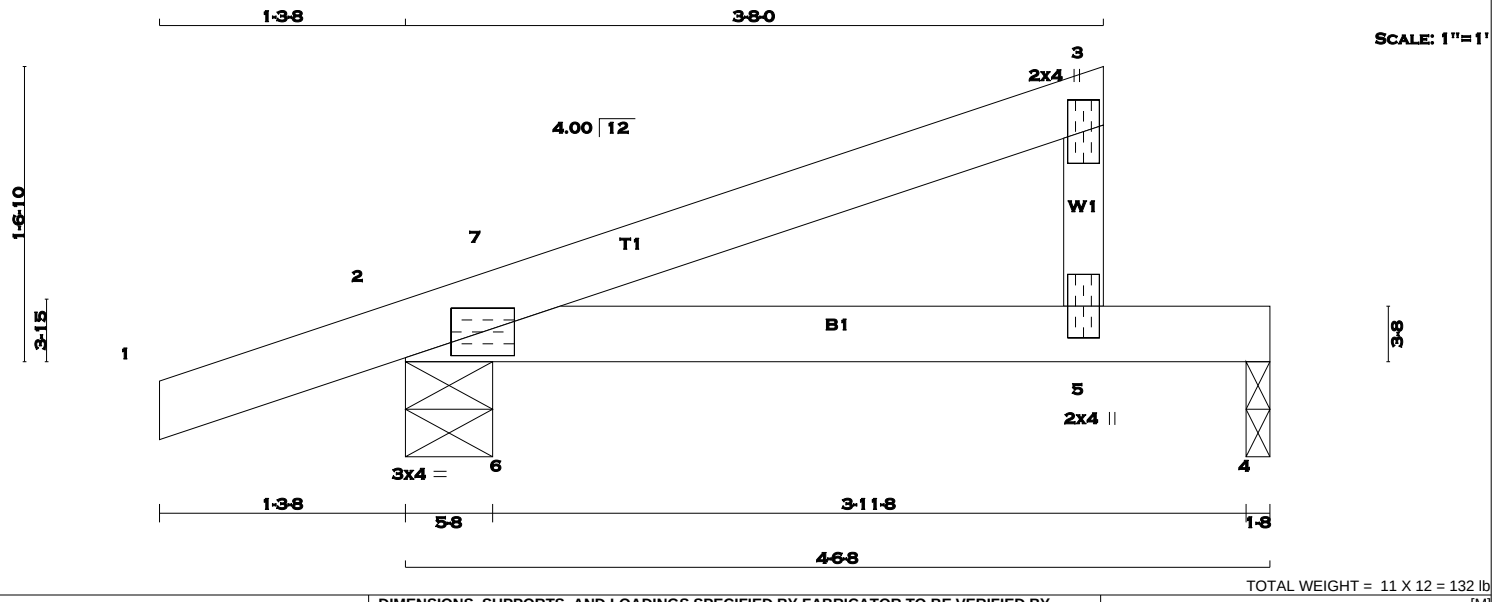

TOTAL WEIGHT = 11 X 12 = 132 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 3	2x4	DRY	No.2	SPF
5 - 3	2x3	DRY	No.2	SPF
2 - 4	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMB1-I	MT20	3.0	4.0		
3	TMV+p	MT20	2.0	4.0		
5	BMV+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
2	312	0	312	103	-167	5-8	5-8
4	154	0	154	0	-69	1-8	1-8

PROVIDE ANCHORAGE AT BEARING JOINT 2 FOR 167 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 4 FOR 150 LBS FACTORED UPLIFT

PROVIDE FOR 103 LBS FACTORED HORIZONTAL REACTION AT JOINT 2

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
2	217	164 / 0	0 / 0	0 / 0	0 / -154	53 / 0	0 / 0
4	110	69 / 0	0 / 0	0 / 0	0 / -75	41 / 0	0 / 0
HORIZONTAL REACTIONS							
2	---	0 / 0	0 / 0	0 / 0	74 / 0	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 2, 4

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 15	-77.3	-77.3	0.10 (1)	10.00	6- 7	-46 / 124	0.00 (1)
2- 7	-74 / 0	-77.3	-77.3	0.07 (1)	6.25			
7- 3	-41 / 5	-77.3	-77.3	0.16 (1)	6.25			
5- 3	-134 / 100	0.0	0.0	0.04 (5)	7.81			
2- 6	-14 / 29	-17.5	-17.5	0.08 (1)	6.25			
6- 5	-14 / 29	-17.5	-17.5	0.18 (1)	6.25			
5- 4	0 / 0	-17.5	-17.5	0.17 (1)	10.00			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	23.3	PSF
		DL =	3.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.0	PSF
TOTAL	LOAD	=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 978 (0.06")

CSI: TC=0.16 (3-7:1) , BC=0.18 (5-6:1) , WB=0.00 (6-7:1) , SSI=0.12 (4-5:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (2) (INPUT = 0.90)
JSI METAL= 0.05 (2) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
T.L. WISE
100083566
PROVINCE OF ONTARIO

March 10, 2017

KOTT

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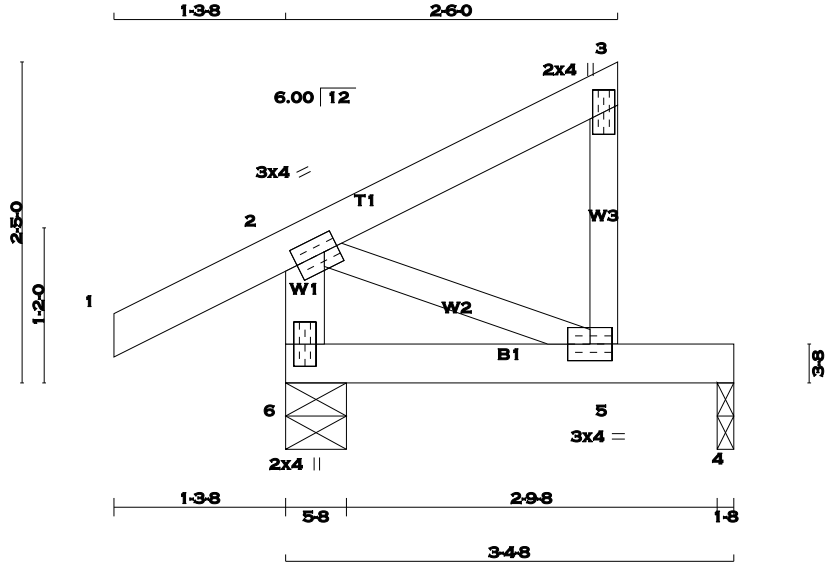

TOTAL WEIGHT = 6 X 13 = 77 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 3	2x4	DRY	No.2	SPF
5 - 3	2x3	DRY	No.2	SPF
6 - 2	2x4	DRY	No.2	SPF
6 - 4	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-t	MT20	3.0	4.0	1.50	1.25
3	TMV+p	MT20	2.0	4.0		
5	BMVW-t	MT20	3.0	4.0		
6	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
6	256	0	256	154	-116	5-8	5-8
4	101	0	109	0	-62	1-8	1-8

PROVIDE ANCHORAGE AT BEARING JOINT 6 FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 4 FOR 150 LBS FACTORED UPLIFT

PROVIDE FOR 154 LBS FACTORED HORIZONTAL REACTION AT JOINT 6

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
6	178	136 / 0	0 / 0	0 / 0	0 / -109	41 / 0	0 / 0
4	72	43 / 0	0 / 0	0 / 0	20 / -63	29 / 0	0 / 0
HORIZONTAL REACTIONS							
6	---	0 / 0	0 / 0	0 / 0	110 / -49	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 6, 4

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (12)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 23	-77.3	-77.3	0.10 (1)	10.00	2- 5	-16 / 121	0.02 (6)
2- 3	-55 / 32	-77.3	-77.3	0.08 (1)	6.25			
5- 3	-103 / 79	0.0	0.0	0.07 (7)	7.81			
6- 2	-207 / 108	0.0	0.0	0.02 (1)	7.81			
6- 5	-136 / 59	-17.5	-17.5	0.11 (1)	6.25			
5- 4	0 / 0	-17.5	-17.5	0.11 (1)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	23.3	PSF
		DL =	3.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.0	PSF
TOTAL	LOAD	=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.10 (1-2:1) , BC=0.11 (5-6:1) , WB=0.02 (2-5:6) , SSI=0.08 (4-5:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (2) (INPUT = 0.90)
JSI METAL= 0.05 (5) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
T.L. WISE
100083566
PROVINCE OF ONTARIO

March 10, 2017

KOTT

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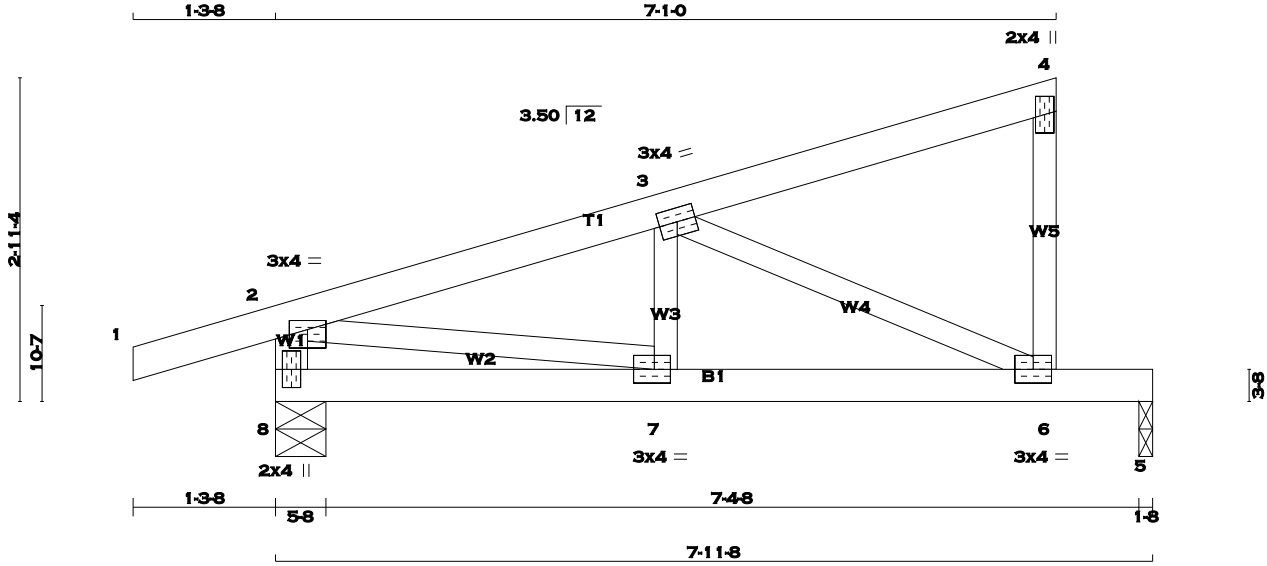


TOTAL WEIGHT = 8 X 28 = 224 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
6 - 4	2x3	DRY	No.2	SPF
8 - 2	2x4	DRY	No.2	SPF
8 - 5	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-p	MT20	3.0	4.0	1.00	2.00
3	TMWW-t	MT20	3.0	4.0		
4	TMV+p	MT20	2.0	4.0		
6	BMVW-t	MT20	3.0	4.0		
7	BMWW-t	MT20	3.0	4.0	1.50	1.75
8	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
8	477	0	477	193	-249	5-8	5-8
5	313	0	313	0	-161	1-8	1-8

PROVIDE ANCHORAGE AT BEARING JOINT 8 FOR 249 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 5 FOR 161 LBS FACTORED UPLIFT

PROVIDE FOR 193 LBS FACTORED HORIZONTAL REACTION AT JOINT 8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
8	332	245 / 0	0 / 0	0 / 0	0 / -234	87 / 0	0 / 0
5	221	147 / 0	0 / 0	0 / 0	0 / -163	75 / 0	0 / 0
HORIZONTAL REACTIONS							
8	---	0 / 0	0 / 0	0 / 0	138 / 0	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 8, 5

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 14	-77.3	-77.3	0.10 (1)	10.00	7- 3	0 / 91	0.03 (11)
2- 3	-581 / 301	-77.3	-77.3	0.16 (5)	6.25	3- 6	-619 / 384	0.16 (1)
3- 4	-50 / 36	-77.3	-77.3	0.12 (5)	6.25	2- 7	-251 / 578	0.13 (1)
6- 4	-108 / 79	0.0	0.0	0.10 (5)	7.81			
8- 2	-467 / 272	0.0	0.0	0.05 (1)	7.81			
8- 7	-180 / 0	-17.5	-17.5	0.10 (1)	6.25			
7- 6	-298 / 564	-17.5	-17.5	0.45 (1)	6.25			
6- 5	0 / 0	-17.5	-17.5	0.36 (1)	10.00			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	23.3	PSF
		DL =	3.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.0	PSF
TOTAL	LOAD	=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.27")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.27")
CALCULATED VERT. DEFL.(TL) = L/ 973 (0.10")

CSI: TC=0.16 (2-3:5) , BC=0.45 (6-7:1) , WB=0.16 (3-6:1) , SSI=0.24 (5-6:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

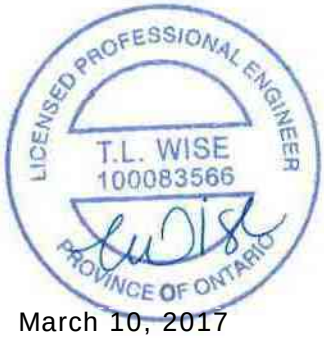
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

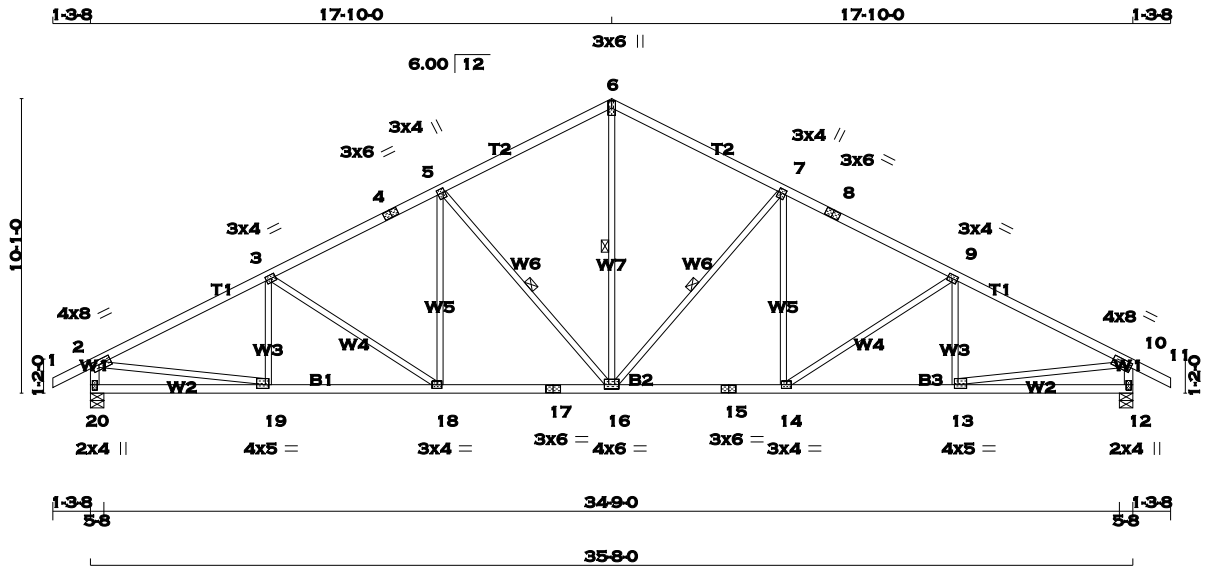
JSI GRIP= 0.86 (6) (INPUT = 0.90)
JSI METAL= 0.21 (7) (INPUT = 1.00)



March 10, 2017

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



TOTAL WEIGHT = 5 X 150 = 751 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 6	2x4	DRY	No.2	SPF
6 - 8	2x4	DRY	No.2	SPF
8 - 11	2x4	DRY	No.2	SPF
20 - 2	2x4	DRY	No.2	SPF
12 - 10	2x4	DRY	No.2	SPF
20 - 17	2x4	DRY	No.2	SPF
17 - 15	2x4	DRY	No.2	SPF
15 - 12	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-t	MT20	4.0	8.0	1.75	3.00
3	TMWW-t	MT20	3.0	4.0	1.50	1.75
4	TS-t	MT20	3.0	6.0		
5	TMWW+t	MT20	3.0	4.0	1.75	0.75
6	TTW+p	MT20	3.0	6.0		
7	TMWW+t	MT20	3.0	4.0	1.75	0.75
8	TS-t	MT20	3.0	6.0		
9	TMWW-t	MT20	3.0	4.0	1.50	1.75
10	TMVW-t	MT20	4.0	8.0	1.75	3.00
12	BMV1+p	MT20	2.0	4.0	2.25	1.00
13	BMWW-t	MT20	4.0	5.0	1.50	1.50
14	BMWW-t	MT20	3.0	4.0		
15	BS-t	MT20	3.0	6.0		
16	BMWWW-t	MT20	4.0	6.0	1.75	3.00
17	BS-t	MT20	3.0	6.0		
18	BMWW-t	MT20	3.0	4.0		
19	BMWW-t	MT20	4.0	5.0	1.50	1.50
20	BMV1+p	MT20	2.0	4.0	2.25	1.00

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

March 10, 2017

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
20	1796	0	1832	251	-804	5-8	5-8
12	1796	0	1832	0	-804	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 20 FOR 804 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 12 FOR 804 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 251 LBS FACTORED HORIZONTAL REACTION AT JOINT 20

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
20	1258	894 / 0	0 / 0	0 / 0	90 / -809	365 / 0	0 / 0
12	1258	894 / 0	0 / 0	0 / 0	90 / -809	365 / 0	0 / 0
HORIZONTAL REACTIONS							
20	---	0 / 0	0 / 0	0 / 0	179 / -179	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 20, 12

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.95 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.03 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 6-16, 7-16, 5-16. DBS = 20-0-0 . CBF = 88 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 23	-77.3	-77.3	0.10 (1)		10.00	16- 6	-568 / 1200	0.38 (8)
2- 3	-2569 / 1107	-77.3	-77.3	0.49 (7)		3.95	16- 7	-752 / 573	0.41 (4)
3- 4	-2290 / 1053	-77.3	-77.3	0.48 (7)		4.24	14- 7	-110 / 286	0.10 (8)
4- 5	-2290 / 1053	-77.3	-77.3	0.48 (7)		4.24	14- 9	-337 / 325	0.31 (4)
5- 6	-1783 / 913	-77.3	-77.3	0.44 (7)		4.68	13- 9	-213 / 209	0.06 (1)
6- 7	-1783 / 913	-77.3	-77.3	0.44 (8)		4.68	5-16	-752 / 573	0.41 (3)
7- 8	-2290 / 1053	-77.3	-77.3	0.48 (8)		4.24	18- 5	-110 / 286	0.10 (7)
8- 9	-2290 / 1053	-77.3	-77.3	0.48 (8)		4.24	3-18	-337 / 325	0.31 (3)
9-10	-2569 / 1107	-77.3	-77.3	0.49 (8)		3.95	19- 3	-213 / 209	0.06 (1)
10-11	0 / 23	-77.3	-77.3	0.10 (1)		10.00	2-19	-861 / 2327	0.53 (7)
20- 2	-1786 / 833	0.0	0.0	0.18 (1)		6.30	13-10	-861 / 2327	0.53 (8)
12-10	-1786 / 833	0.0	0.0	0.18 (1)		6.30			
20-19	-234 / 268	-17.5	-17.5	0.14 (11)		6.25			
19-18	-1087 / 2365	-17.5	-17.5	0.42 (1)		6.03			
18-17	-816 / 2085	-17.5	-17.5	0.40 (1)		6.25			
17-16	-816 / 2085	-17.5	-17.5	0.40 (1)		6.25			
16-15	-621 / 2044	-17.5	-17.5	0.40 (1)		6.25			
15-14	-621 / 2044	-17.5	-17.5	0.40 (1)		6.25			
14-13	-836 / 2297	-17.5	-17.5	0.42 (1)		6.25			
13-12	-9 / 18	-17.5	-17.5	0.14 (11)		10.00			

RECEIVED
TOWN OF MILTON
MAR 29, 2017
17-4978
BUILDING DIVISION

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (1.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.26")

CSI: TC=0.49 (9-10:8) , BC=0.42 (13-14:1) , WB=0.53 (10-13:8) , SSI=0.19 (9-10:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

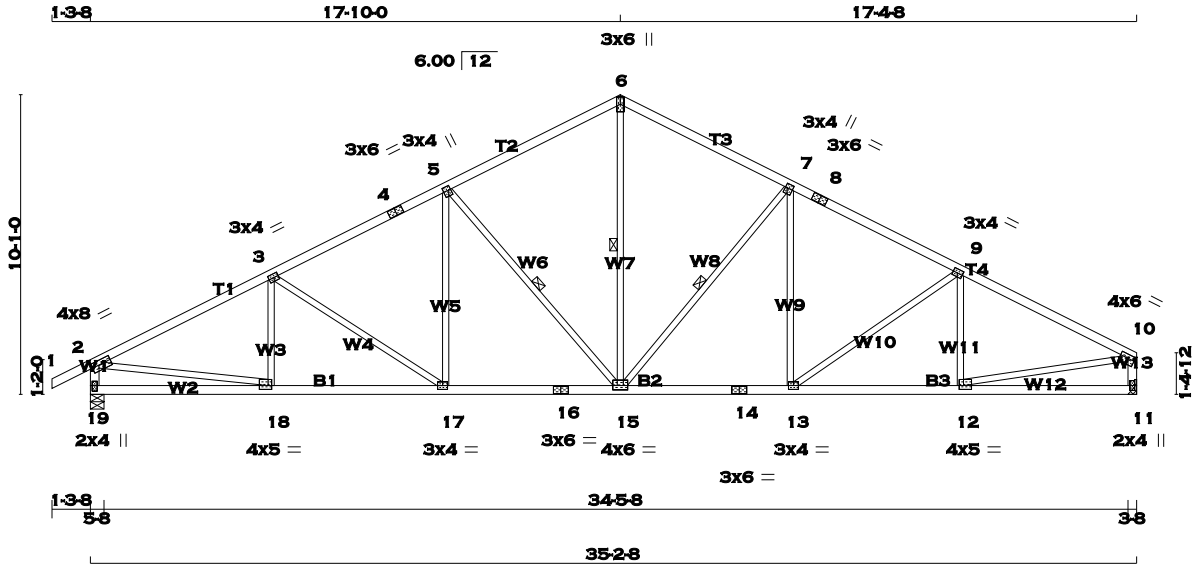
JSI GRIP= 0.88 (16) (INPUT = 0.90)
JSI METAL= 0.69 (13) (INPUT = 1.00)

CONTINUED ON PAGE 2

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



LUMBER

N. L. G. A. RULES

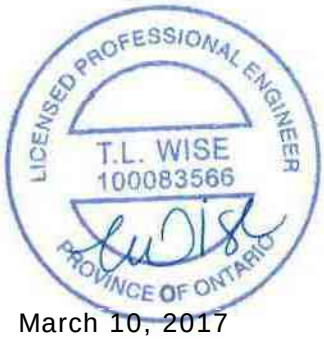
CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 6	2x4	DRY	No.2	SPF
6 - 8	2x4	DRY	No.2	SPF
8 - 10	2x4	DRY	No.2	SPF
19 - 2	2x4	DRY	No.2	SPF
11 - 10	2x4	DRY	No.2	SPF
19 - 16	2x4	DRY	No.2	SPF
16 - 14	2x4	DRY	No.2	SPF
14 - 11	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-t	MT20	4.0	8.0	1.75	3.00
3	TMWW-t	MT20	3.0	4.0	1.50	1.75
4	TS-t	MT20	3.0	6.0		
5	TMWW+t	MT20	3.0	4.0	1.75	0.75
6	TTW+p	MT20	3.0	6.0		
7	TMWW+t	MT20	3.0	4.0	1.75	0.75
8	TS-t	MT20	3.0	6.0		
9	TMWW-t	MT20	3.0	4.0	1.50	1.75
10	TMVW-t	MT20	4.0	6.0	1.50	2.75
11	BMV1+p	MT20	2.0	4.0		
12	BMWW-t	MT20	4.0	5.0	1.50	1.75
13	BMWW-t	MT20	3.0	4.0		
14	BS-t	MT20	3.0	6.0		
15	BMWWW-t	MT20	4.0	6.0	1.75	3.00
16	BS-t	MT20	3.0	6.0		
17	BMWW-t	MT20	3.0	4.0		
18	BMWW-t	MT20	4.0	5.0	1.50	1.50
19	BMV1+p	MT20	2.0	4.0	2.25	1.00

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.



March 10, 2017

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
19	1775	0	1811	269	-796	5-8	5-8
11	1670	0	1708	0	-728	HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-8	

PROVIDE ANCHORAGE AT BEARING JOINT 19 FOR 796 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 11 FOR 728 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 269 LBS FACTORED HORIZONTAL REACTION AT JOINT 19

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
19	1243	883 / 0	0 / 0	0 / 0	90 / -800	360 / 0	0 / 0
11	1172	820 / 0	0 / 0	0 / 0	95 / -746	352 / 0	0 / 0
HORIZONTAL REACTIONS							
19	---	0 / 0	0 / 0	0 / 0	192 / -157	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 19

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.02 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 5-15, 6-15, 7-15. DBS = 20-0-0 . CBF = 89 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 23	-77.3	-77.3	0.10 (1)	10.00	18- 3	-208 / 207	0.05 (1)
2- 3	-2531 / 1093	-77.3	-77.3	0.48 (7)	3.98	3-17	-341 / 327	0.32 (3)
3- 4	-2247 / 1037	-77.3	-77.3	0.48 (7)	4.28	17- 5	-110 / 289	0.10 (7)
4- 5	-2247 / 1037	-77.3	-77.3	0.48 (7)	4.28	5-15	-755 / 573	0.41 (3)
5- 6	-1737 / 892	-77.3	-77.3	0.44 (7)	4.74	15- 6	-556 / 1165	0.37 (7)
6- 7	-1738 / 898	-77.3	-77.3	0.42 (8)	4.76	15- 7	-693 / 543	0.37 (4)
7- 8	-2191 / 1010	-77.3	-77.3	0.46 (8)	4.34	13- 7	-94 / 240	0.08 (8)
8- 9	-2191 / 1010	-77.3	-77.3	0.46 (8)	4.34	13- 9	-249 / 280	0.23 (4)
9-10	-2384 / 1023	-77.3	-77.3	0.46 (8)	4.12	12- 9	-281 / 235	0.08 (1)
19- 2	-1764 / 825	0.0	0.0	0.17 (1)	6.33	2-18	-849 / 2293	0.53 (7)
11-10	-1663 / 756	0.0	0.0	0.17 (1)	6.48	12-10	-825 / 2171	0.49 (8)
19-18	-253 / 226	-17.5	-17.5	0.14 (11)	6.25			
18-17	-1093 / 2326	-17.5	-17.5	0.42 (1)	6.02			
17-16	-820 / 2042	-17.5	-17.5	0.39 (1)	6.25			
16-15	-820 / 2042	-17.5	-17.5	0.39 (1)	6.25			
15-14	-608 / 1952	-17.5	-17.5	0.38 (1)	6.25			
14-13	-608 / 1952	-17.5	-17.5	0.38 (1)	6.25			
13-12	-790 / 2126	-17.5	-17.5	0.40 (1)	6.25			
12-11	-11 / 23	-17.5	-17.5	0.14 (11)	6.25			

RECEIVED
TOWN OF MILTON
MAR 29, 2017
17-4978
BUILDING DIVISION

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.17")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (1.17")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.48 (2-3:7) , BC=0.42 (17-18:1) , WB=0.53 (2-18:7) , SSI=0.19 (2-3:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

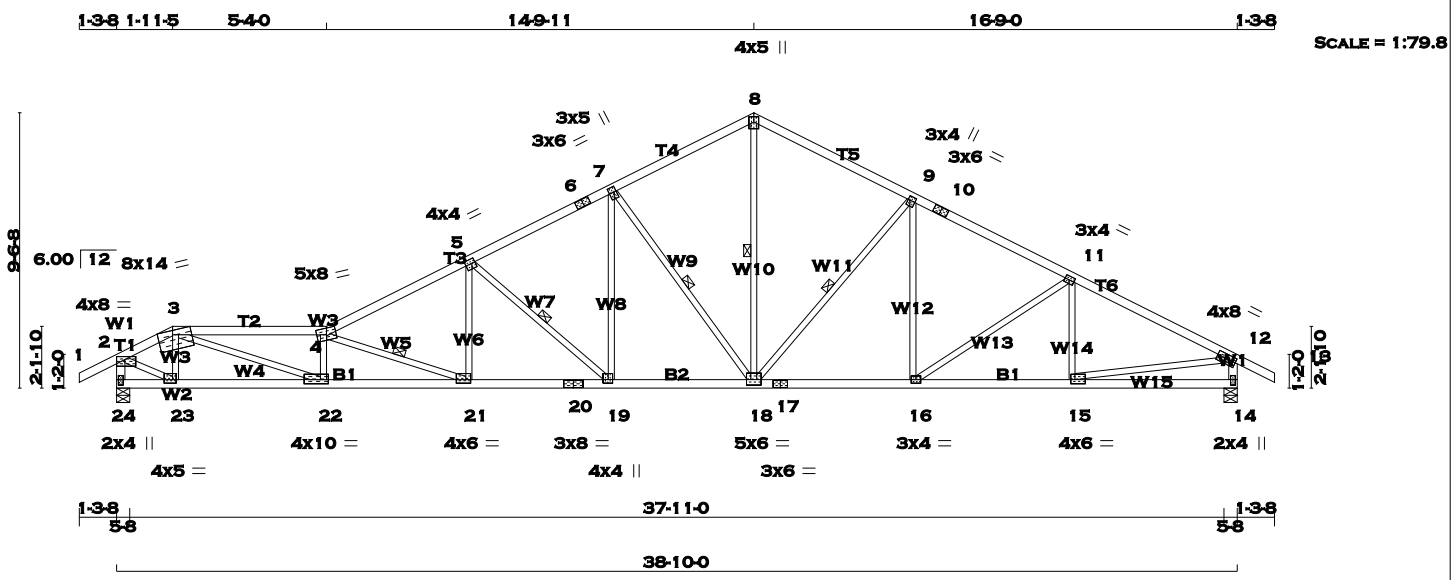
JSI GRIP= 0.90 (12) (INPUT = 0.90)
JSI METAL= 0.68 (18) (INPUT = 1.00)

CONTINUED ON PAGE 2

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



TOTAL WEIGHT = 162 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 3	2x4	DRY	No.2	SPF
3 - 4	2x4	DRY	No.2	SPF
4 - 6	2x4	DRY	No.2	SPF
6 - 8	2x4	DRY	No.2	SPF
8 - 10	2x4	DRY	No.2	SPF
10 - 13	2x4	DRY	No.2	SPF
24 - 2	2x4	DRY	No.2	SPF
14 - 12	2x4	DRY	No.2	SPF
24 - 20	2x4	DRY	1650F 1.5E	SPF
20 - 17	2x4	DRY	1650F 1.5E	SPF
17 - 14	2x4	DRY	1650F 1.5E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

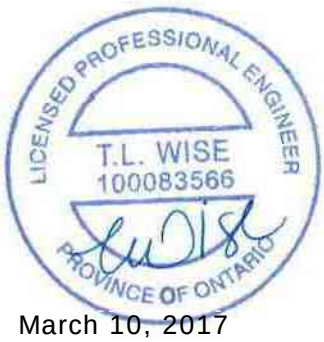
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-p	MT20	4.0	8.0	1.25	3.50
3	TTWW-m	MT20	8.0	14.0	Edge	6.25
4	TTWW-m	MT20	5.0	8.0	2.75	4.00
5	TMWW-t	MT20	4.0	4.0	1.75	1.25
6	TS-t	MT20	3.0	6.0		
7	TMWW+t	MT20	3.0	5.0	2.25	0.75
8	TTW+p	MT20	4.0	5.0	2.25	2.00
9	TMWW+t	MT20	3.0	4.0	1.75	0.75
10	TS-t	MT20	3.0	6.0		
11	TMWW-t	MT20	3.0	4.0	1.50	1.75
12	TMVW-t	MT20	4.0	8.0	1.75	3.00
14	BMV1+p	MT20	2.0	4.0	2.25	1.00
15	BMWW-t	MT20	4.0	6.0	1.75	1.75
16	BMWW-t	MT20	3.0	4.0		
17	BS-t	MT20	3.0	6.0		
18	BMWWW-t	MT20	5.0	6.0	2.25	3.00
19	BMWW+t	MT20	4.0	4.0	1.50	1.75
20	BS-t	MT20	3.0	8.0		
21	BMWW-t	MT20	4.0	6.0	1.50	2.00
22	BMWW-t	MT20	4.0	10.0	1.75	3.25
23	BMWW-t	MT20	4.0	5.0	1.50	1.50
24	BMV1+p	MT20	2.0	4.0	2.25	1.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.



March 10, 2017

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
24	1947	0	1995	237	-912	5-8	5-8
14	1947	0	1980	0	-842	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 24 FOR 912 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 14 FOR 842 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 237 LBS FACTORED HORIZONTAL REACTION AT JOINT 24

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
24	1364	967 / 0	0 / 0	0 / 0	121 / -906	396 / 0	0 / 0
14	1364	967 / 0	0 / 0	0 / 0	84 / -856	396 / 0	0 / 0
HORIZONTAL REACTIONS							
24	---	0 / 0	0 / 0	0 / 0	169 / -169	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 24, 14

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.34 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 4.69 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 4-21. DBS = 6-0-0 . CBF = 89 LBS.
1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 5-19. DBS = 10-0-0 . CBF = 83 LBS.
1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 7-18. DBS = 12-0-0 . CBF = 93 LBS.
1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 8-18. DBS = 18-0-0 . CBF = 89 LBS.
1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 9-18. DBS = 20-0-0 . CBF = 78 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 23	-77.3	-77.3	0.10 (1)	10.00	23- 3	-677 / 363	0.10 (1)
2- 3	-2248 / 1021	-77.3	-77.3	0.19 (7)	4.56	3-22	-1795 / 4201	0.92 (1)
3- 4	-5934 / 2619	-77.3	-77.3	0.78 (1)	2.34	22- 4	-1387 / 699	0.20 (1)
4- 5	-4146 / 1825	-77.3	-77.3	0.48 (7)	3.20	4-21	-2445 / 1167	0.53 (1)
5- 6	-3007 / 1379	-77.3	-77.3	0.42 (7)	3.83	21- 5	-364 / 984	0.22 (1)
6- 7	-3007 / 1379	-77.3	-77.3	0.42 (7)	3.83	5-19	-1387 / 772	0.39 (1)
7- 8	-2183 / 1064	-77.3	-77.3	0.37 (7)	4.41	19- 7	-438 / 980	0.37 (7)
8- 9	-2191 / 1087	-77.3	-77.3	0.42 (8)	4.34	7-18	-1293 / 789	0.59 (3)
9-10	-2623 / 1145	-77.3	-77.3	0.45 (8)	4.02	18- 8	-757 / 1608	0.44 (7)
10-11	-2623 / 1145	-77.3	-77.3	0.45 (8)	4.02	18- 9	-666 / 523	0.32 (4)
11-12	-2811 / 1170	-77.3	-77.3	0.45 (8)	3.83	16- 9	-89 / 226	0.07 (8)
12-13	0 / 23	-77.3	-77.3	0.10 (1)	10.00	16-11	-238 / 279	0.19 (4)
24- 2	-2031 / 931	0.0	0.0	0.20 (1)	5.99	15-11	-274 / 221	0.07 (1)
14-12	-1935 / 868	0.0	0.0	0.19 (1)	6.09	2-23	-896 / 2168	0.48 (1)
						15-12	-9[illegible]4 / 2548	0.57 (1)
24-23	-220 / 254	-17.5	-17.5	0.12 (1)	6.25			
23-22	-1026 / 2019	-17.5	-17.5	0.36 (1)	6.25			
22-21	-2759 / 6061	-17.5	-17.5	0.82 (1)	4.69			
21-20	-1666 / 3770	-17.5	-17.5	0.49 (1)	5.74			
20-19	-1666 / 3770	-17.5	-17.5	0.49 (1)	5.74			
19-18	-1084 / 2723	-17.5	-17.5	0.37 (1)	6.25			
18-17	-773 / 2353	-17.5	-17.5	0.33 (1)	6.25			
17-16	-773 / 2353	-17.5	-17.5	0.33 (1)	6.25			
16-15	-897 / 2515	-17.5	-17.5	0.33 (1)	6.25			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.29")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.42")
ALLOWABLE DEFL.(TL)= L/360 (1.29")
CALCULATED VERT. DEFL.(TL) = L/ 638 (0.73")

CSI: TC=0.78 (3-4:1) , BC=0.82 (21-22:1) , WB=0.92 (3-22:1) , SSI=0.18 (11-12:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (3) (INPUT = 0.90)
JSI METAL= 0.97 (20) (INPUT = 1.00)

CONTINUED ON PAGE 2

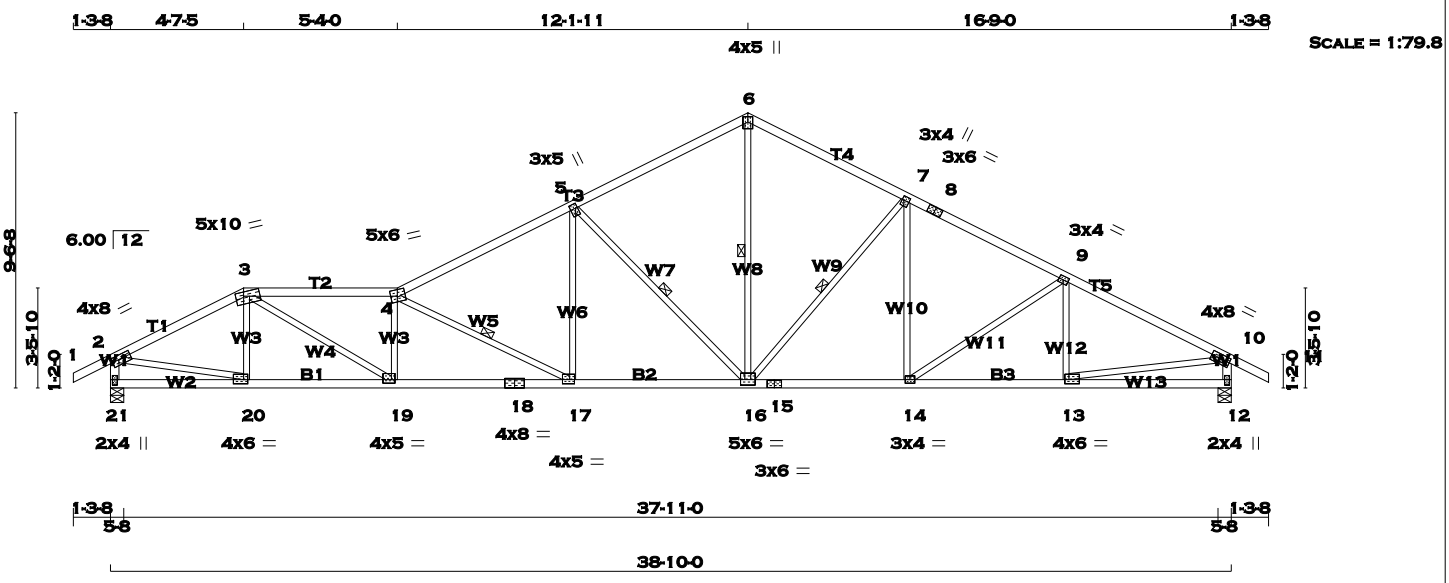
LOADING
TOTAL LOAD CASES: (11)

CHORDS					WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO						
15-14	-9 / 18	-17.5 -17.5	0.10	(11)	10.00			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



TOTAL WEIGHT = 160 lb

[M][F]

LUMBER

N. L. G. A. RULES

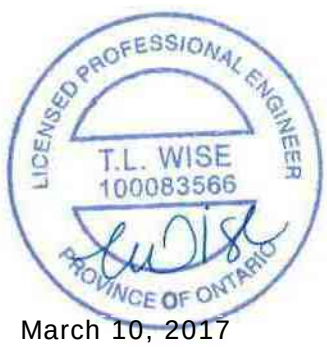
CHORDS	SIZE		LUMBER	DESCR.
1 - 3	2x4	DRY	No.2	SPF
3 - 4	2x4	DRY	No.2	SPF
4 - 6	2x4	DRY	No.2	SPF
6 - 8	2x4	DRY	No.2	SPF
8 - 11	2x4	DRY	No.2	SPF
21 - 2	2x4	DRY	No.2	SPF
12 - 10	2x4	DRY	No.2	SPF
21 - 18	2x4	DRY	No.2	SPF
18 - 15	2x4	DRY	No.2	SPF
15 - 12	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-t	MT20	4.0	8.0	1.75	3.00
3	TTWW-m	MT20	5.0	10.0	1.75	3.25
4	TTWW-m	MT20	5.0	6.0	2.75	2.75
5	TMWW+t	MT20	3.0	5.0	2.00	0.75
6	TTW+p	MT20	4.0	5.0	2.25	2.00
7	TMWW+t	MT20	3.0	4.0	1.75	0.75
8	TS-t	MT20	3.0	6.0		
9	TMWW-t	MT20	3.0	4.0	1.50	1.75
10	TMVW-t	MT20	4.0	8.0	1.75	3.00
12	BMV1+p	MT20	2.0	4.0	2.25	1.00
13	BMWW-t	MT20	4.0	6.0	1.75	1.75
14	BMWW-t	MT20	3.0	4.0		
15	BS-t	MT20	3.0	6.0		
16	BMWWW-t	MT20	5.0	6.0	2.25	3.00
17	BMWW-t	MT20	4.0	5.0	1.75	2.00
18	BS-t	MT20	4.0	8.0		
19	BMWW-t	MT20	4.0	5.0	1.50	1.50
20	BMWW-t	MT20	4.0	6.0	1.75	1.75
21	BMV1+p	MT20	2.0	4.0	2.25	1.00

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.



March 10, 2017

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
21	1947	0	1995	237	-912	5-8	5-8
12	1947	0	1980	0	-842	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 21 FOR 912 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 12 FOR 842 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 237 LBS FACTORED HORIZONTAL REACTION AT JOINT 21

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
21	1364	967 / 0	0 / 0	0 / 0	121 / -906	396 / 0	0 / 0
12	1364	967 / 0	0 / 0	0 / 0	84 / -856	396 / 0	0 / 0
HORIZONTAL REACTIONS							
21	---	0 / 0	0 / 0	0 / 0	169 / -169	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 21, 12

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 4.79 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 4-17. DBS = 8-0-0 . CBF = 81 LBS.
1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 5-16. DBS = 10-0-0 . CBF = 85 LBS.
1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 6-16. DBS = 18-0-0 . CBF = 87 LBS.
1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 7-16. DBS = 20-0-0 . CBF = 78 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
1- 2	0 / 23	-77.3	-77.3	0.10 (1)	10.00
2- 3	-2740 / 1228	-77.3	-77.3	0.42 (7)	3.99
3- 4	-4366 / 1959	-77.3	-77.3	0.55 (1)	3.00
4- 5	-3237 / 1446	-77.3	-77.3	0.63 (7)	3.47
5- 6	-2192 / 1051	-77.3	-77.3	0.57 (7)	4.20
6- 7	-2192 / 1086	-77.3	-77.3	0.42 (8)	4.34
7- 8	-2623 / 1145	-77.3	-77.3	0.45 (8)	4.02
8- 9	-2623 / 1145	-77.3	-77.3	0.45 (8)	4.02
9-10	-2812 / 1170	-77.3	-77.3	0.45 (8)	3.83
10-11	0 / 23	-77.3	-77.3	0.10 (1)	10.00
21- 2	-1962 / 935	0.0	0.0	0.19 (1)	6.07
12-10	-1936 / 869	0.0	0.0	0.19 (1)	6.09
21-20	-220 / 254	-17.5	-17.5	0.09 (11)	6.25
20-19	-1179 / 2486	-17.5	-17.5	0.44 (1)	5.86
19-18	-1987 / 4445	-17.5	-17.5	0.77 (1)	4.79
18-17	-1987 / 4445	-17.5	-17.5	0.77 (1)	4.79
17-16	-1225 / 2958	-17.5	-17.5	0.53 (1)	5.79
16-15	-774 / 2353	-17.5	-17.5	0.44 (1)	6.25
15-14	-774 / 2353	-17.5	-17.5	0.44 (1)	6.25
14-13	-897 / 2515	-17.5	-17.5	0.45 (1)	6.25
13-12	-9 / 18	-17.5	-17.5	0.12 (11)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
20- 3	-341 / 239	0.07 (1)
3-19	-924 / 2249	0.57 (7)
19- 4	-1085 / 557	0.22 (1)
4-17	-1677 / 860	0.53 (1)
17- 5	-321 / 892	0.22 (7)
5-16	-1425 / 868	0.71 (3)
16- 6	-732 / 1583	0.43 (7)
16- 7	-665 / 525	0.32 (4)
14- 7	-91 / 226	0.07 (8)
14- 9	-238 / 279	0.19 (4)
13- 9	-274 / 221	0.07 (1)
2-20	-983 / 2479	0.55 (1)
13-10	-924 / 2549	0.57 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.29")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.30")
ALLOWABLE DEFL.(TL)= L/360 (1.29")
CALCULATED VERT. DEFL.(TL) = L/ 871 (0.54")

CSI: TC=0.63 (4-5:7) , BC=0.77 (17-19:1) , WB=0.71 (5-16:3) , SSI=0.20 (4-5:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

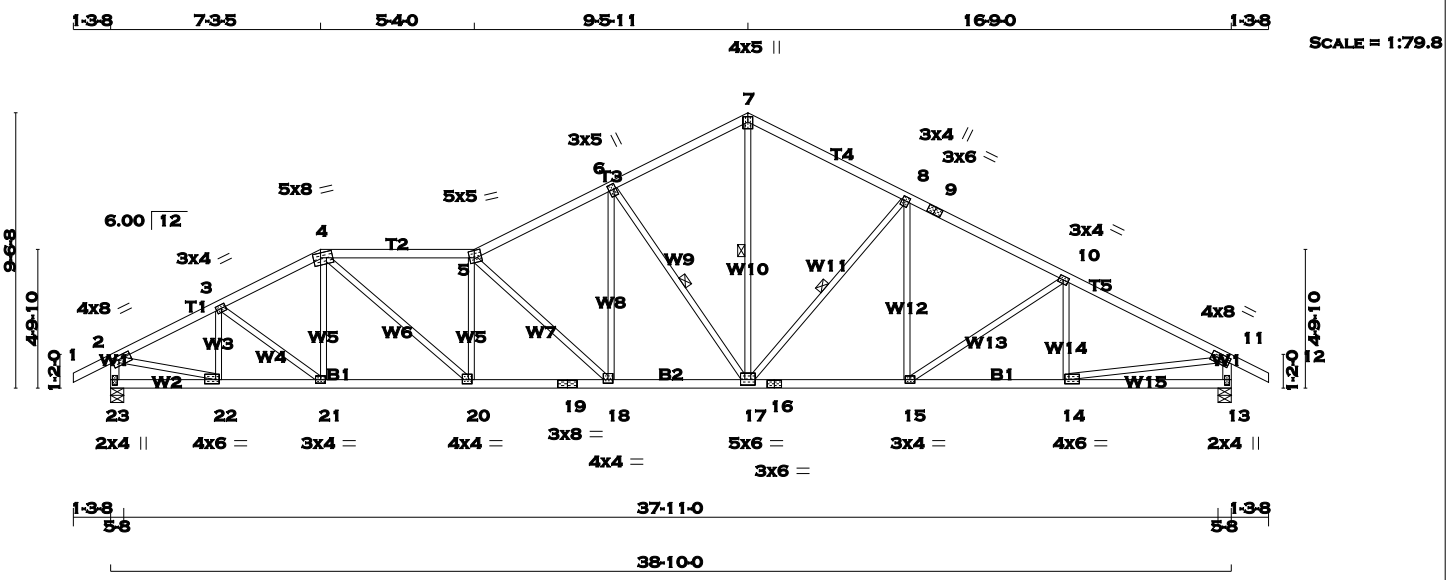
JSI GRIP= 0.90 (3) (INPUT = 0.90)
JSI METAL= 0.87 (18) (INPUT = 1.00)

CONTINUED ON PAGE 2

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



TOTAL WEIGHT = 167 lb [M][F]

LUMBER

N. L. G. A. RULES

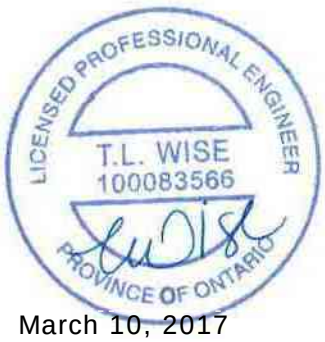
CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 5	2x4	DRY	No.2	SPF
5 - 7	2x4	DRY	No.2	SPF
7 - 9	2x4	DRY	No.2	SPF
9 - 12	2x4	DRY	No.2	SPF
23 - 2	2x4	DRY	No.2	SPF
13 - 11	2x4	DRY	No.2	SPF
23 - 19	2x4	DRY	No.2	SPF
19 - 16	2x4	DRY	No.2	SPF
16 - 13	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-t	MT20	4.0	8.0	1.50	3.00
3	TMWW-t	MT20	3.0	4.0	1.50	1.75
4	TTWW-m	MT20	5.0	8.0	2.00	3.00
5	TTWW-m	MT20	5.0	5.0	3.00	2.00
6	TMWW+t	MT20	3.0	5.0	2.25	0.75
7	TTW+p	MT20	4.0	5.0	2.25	2.00
8	TMWW+t	MT20	3.0	4.0	1.75	0.75
9	TS-t	MT20	3.0	6.0		
10	TMWW-t	MT20	3.0	4.0	1.50	1.75
11	TMVW-t	MT20	4.0	8.0	1.75	3.00
13	BMV1+p	MT20	2.0	4.0	2.25	1.00
14	BMWW-t	MT20	4.0	6.0	1.75	1.75
15	BMWW-t	MT20	3.0	4.0		
16	BS-t	MT20	3.0	6.0		
17	BMWWW-t	MT20	5.0	6.0	2.25	3.00
18	BMWW-t	MT20	4.0	4.0	1.50	2.00
19	BS-t	MT20	3.0	8.0		
20	BMWW-t	MT20	4.0	4.0	1.75	1.50
21	BMWW-t	MT20	3.0	4.0		
22	BMWW-t	MT20	4.0	6.0	1.75	1.50
23	BMV1+p	MT20	2.0	4.0	2.25	1.00

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.



March 10, 2017

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
23	1947	0	1995	237	-912	5-8	5-8
13	1947	0	1980	0	-842	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 23 FOR 912 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 13 FOR 842 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 237 LBS FACTORED HORIZONTAL REACTION AT JOINT 23

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
23	1364	967 / 0	0 / 0	0 / 0	121 / -906	396 / 0	0 / 0
13	1364	967 / 0	0 / 0	0 / 0	84 / -856	396 / 0	0 / 0
HORIZONTAL REACTIONS							
23	---	0 / 0	0 / 0	0 / 0	169 / -169	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 23, 13

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.41 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 5.28 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 6-17. DBS = 12-0-0 . CBF = 93 LBS.
1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 7-17. DBS = 18-0-0 . CBF = 89 LBS.
1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 8-17. DBS = 20-0-0 . CBF = 78 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
1- 2	0 / 23	-77.3	-77.3	0.10 (1)	10.00
2- 3	-2647 / 1217	-77.3	-77.3	0.31 (7)	4.16
3- 4	-2804 / 1323	-77.3	-77.3	0.32 (7)	4.07
4- 5	-3548 / 1631	-77.3	-77.3	0.49 (7)	3.41
5- 6	-2953 / 1353	-77.3	-77.3	0.44 (7)	3.84
6- 7	-2186 / 1071	-77.3	-77.3	0.40 (7)	4.41
7- 8	-2191 / 1087	-77.3	-77.3	0.42 (8)	4.34
8- 9	-2623 / 1145	-77.3	-77.3	0.45 (8)	4.02
9-10	-2623 / 1145	-77.3	-77.3	0.45 (8)	4.02
10-11	-2812 / 1170	-77.3	-77.3	0.45 (8)	3.83
11-12	0 / 23	-77.3	-77.3	0.10 (1)	10.00
23- 2	-1958 / 926	0.0	0.0	0.19 (1)	6.08
13-11	-1936 / 869	0.0	0.0	0.19 (1)	6.09
23-22	-220 / 254	-17.5	-17.5	0.07 (8)	6.25
22-21	-1205 / 2428	-17.5	-17.5	0.43 (1)	5.83
21-20	-1149 / 2536	-17.5	-17.5	0.44 (1)	5.93
20-19	-1560 / 3605	-17.5	-17.5	0.61 (1)	5.28
19-18	-1560 / 3605	-17.5	-17.5	0.61 (1)	5.28
18-17	-1069 / 2694	-17.5	-17.5	0.48 (1)	6.10
17-16	-773 / 2353	-17.5	-17.5	0.44 (1)	6.25
16-15	-773 / 2353	-17.5	-17.5	0.44 (1)	6.25
15-14	-897 / 2515	-17.5	-17.5	0.45 (1)	6.25
14-13	-9 / 18	-17.5	-17.5	0.12 (11)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
22- 3	-497 / 286	0.09 (1)
3-21	-8 / 152	0.03 (1)
21- 4	0 / 61	0.02 (11)
4-20	-530 / 1387	0.45 (7)
20- 5	-824 / 416	0.28 (1)
5-18	-1257 / 678	0.96 (3)
18- 6	-408 / 943	0.36 (7)
6-17	-1291 / 793	0.59 (3)
17- 7	-772 / 1623	0.45 (7)
17- 8	-666 / 523	0.32 (4)
15- 8	-89 / 227	0.07 (8)
15-10	-238 / 279	0.19 (4)
14-10	-274 / 220	0.07 (1)
2-22	-1011 / 2431	0.53 (1)
14-11	-924 / 2548	0.57 (1)

RECEIVED
TOWN OF MILTON
MAR 29, 2017
17-4978
BUILDING DIVISION

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.29")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.24")
ALLOWABLE DEFL.(TL)= L/360 (1.29")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.43")

CSI: TC=0.49 (4-5:7) , BC=0.61 (18-20:1) , WB=0.96 (5-18:3) , SSI=0.18 (10-11:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

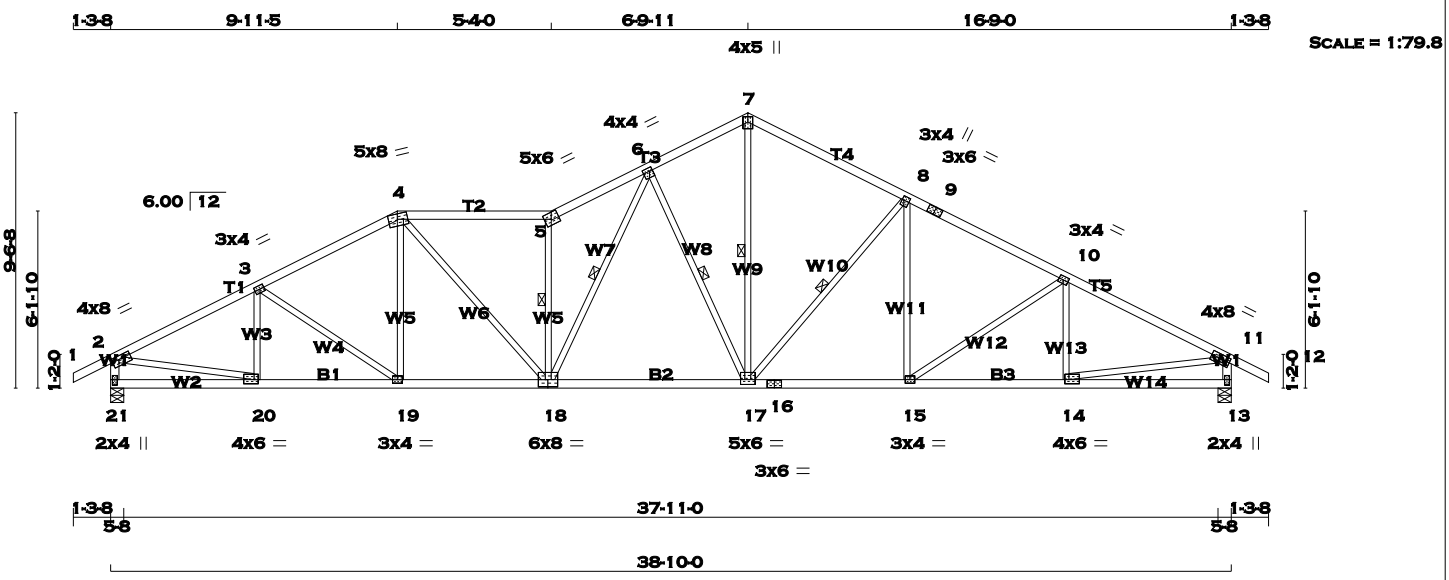
JSI GRIP= 0.90 (20) (INPUT = 0.90)
JSI METAL= 0.96 (19) (INPUT = 1.00)

CONTINUED ON PAGE 2

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



LUMBER

N. L. G. A. RULES

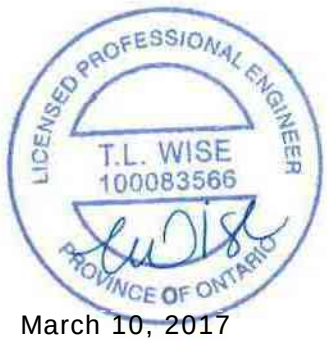
CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 5	2x4	DRY	No.2	SPF
5 - 7	2x4	DRY	No.2	SPF
7 - 9	2x4	DRY	No.2	SPF
9 - 12	2x4	DRY	No.2	SPF
21 - 2	2x4	DRY	No.2	SPF
13 - 11	2x4	DRY	No.2	SPF
21 - 18	2x4	DRY	No.2	SPF
18 - 16	2x4	DRY	No.2	SPF
16 - 13	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-t	MT20	4.0	8.0	1.50	3.00
3	TMWW-t	MT20	3.0	4.0	1.50	1.75
4	TTWW-m	MT20	5.0	8.0	2.25	3.75
5	TTW-h	MT20	5.0	6.0	2.75	3.25
6	TMWW-t	MT20	4.0	4.0	1.50	1.25
7	TTW+p	MT20	4.0	5.0	2.25	2.00
8	TMWW+t	MT20	3.0	4.0	1.75	0.75
9	TS-t	MT20	3.0	6.0		
10	TMWW-t	MT20	3.0	4.0	1.50	1.75
11	TMVW-t	MT20	4.0	8.0	1.75	3.00
13	BMV1+p	MT20	2.0	4.0	2.25	1.00
14	BMWW-t	MT20	4.0	6.0	1.75	1.75
15	BMWW-t	MT20	3.0	4.0		
16	BS-t	MT20	3.0	6.0		
17	BMWWW-t	MT20	5.0	6.0	2.00	3.00
18	BSWWW-l	MT20	6.0	8.0		
19	BMWW-t	MT20	3.0	4.0		
20	BMWW-t	MT20	4.0	6.0	1.75	1.75
21	BMV1+p	MT20	2.0	4.0	2.25	1.00

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.



March 10, 2017

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
21	1947	0	1995	237	-912	5-8	5-8
13	1947	0	1980	0	-842	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 21 FOR 912 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 13 FOR 842 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 237 LBS FACTORED HORIZONTAL REACTION AT JOINT 21

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
21	1364	967 / 0	0 / 0	0 / 0	121 / -906	396 / 0	0 / 0
13	1364	967 / 0	0 / 0	0 / 0	84 / -856	396 / 0	0 / 0
HORIZONTAL REACTIONS							
21	---	0 / 0	0 / 0	0 / 0	169 / -169	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 21, 13

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.71 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 5.74 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 5-18. DBS = 8-0-0 . CBF = 89 LBS.
1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 6-18, 8-17. DBS = 20-0-0 . CBF = 85 LBS.
1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 6-17. DBS = 12-0-0 . CBF = 86 LBS.
1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 7-17. DBS = 18-0-0 . CBF = 92 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
1- 2	0 / 23	-77.3	-77.3	0.10 (1)	10.00
2- 3	-2807 / 1283	-77.3	-77.3	0.45 (7)	3.92
3- 4	-2705 / 1291	-77.3	-77.3	0.46 (7)	4.00
4- 5	-2984 / 1406	-77.3	-77.3	0.46 (7)	3.74
5- 6	-3363 / 1642	-77.3	-77.3	0.36 (7)	3.71
6- 7	-2185 / 1089	-77.3	-77.3	0.28 (7)	4.52
7- 8	-2194 / 1086	-77.3	-77.3	0.42 (8)	4.33
8- 9	-2622 / 1145	-77.3	-77.3	0.45 (8)	4.02
9-10	-2622 / 1145	-77.3	-77.3	0.45 (8)	4.02
10-11	-2812 / 1170	-77.3	-77.3	0.45 (8)	3.83
11-12	0 / 23	-77.3	-77.3	0.10 (1)	10.00
21- 2	-1953 / 934	0.0	0.0	0.19 (1)	6.09
13-11	-1936 / 868	0.0	0.0	0.19 (1)	6.09
21-20	-220 / 254	-17.5	-17.5	0.09 (11)	6.25
20-19	-1248 / 2576	-17.5	-17.5	0.45 (1)	5.74
19-18	-1049 / 2441	-17.5	-17.5	0.43 (1)	6.11
18-17	-935 / 2455	-17.5	-17.5	0.48 (1)	6.25
17-16	-774 / 2352	-17.5	-17.5	0.47 (1)	6.25
16-15	-774 / 2352	-17.5	-17.5	0.47 (1)	6.25
15-14	-897 / 2515	-17.5	-17.5	0.45 (1)	6.25
14-13	-9 / 18	-17.5	-17.5	0.12 (11)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
20- 3	-340 / 254	0.07 (1)
3-19	-169 / 243	0.10 (3)
19- 4	-89 / 191	0.05 (7)
4-18	-373 / 885	0.45 (8)
18- 5	-1827 / 979	0.44 (1)
18- 6	-754 / 1411	0.34 (7)
6-17	-1194 / 773	0.53 (3)
17- 7	-811 / 1668	0.47 (7)
17- 8	-661 / 527	0.31 (4)
15- 8	-92 / 221	0.07 (8)
15-10	-240 / 278	0.19 (4)
14-10	-272 / 221	0.07 (1)
2-20	-1042 / 2554	0.56 (1)
14-11	-924 / 2549	0.57 (1)

RECEIVED
TOWN OF MILTON
MAR 29, 2017
17-4978
BUILDING DIVISION

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.29")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.23")
ALLOWABLE DEFL.(TL)= L/360 (1.29")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.43")

CSI: TC=0.46 (4-5:7) , BC=0.48 (17-18:1) , WB=0.57 (11-14:1) , SSI=0.18 (10-11:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

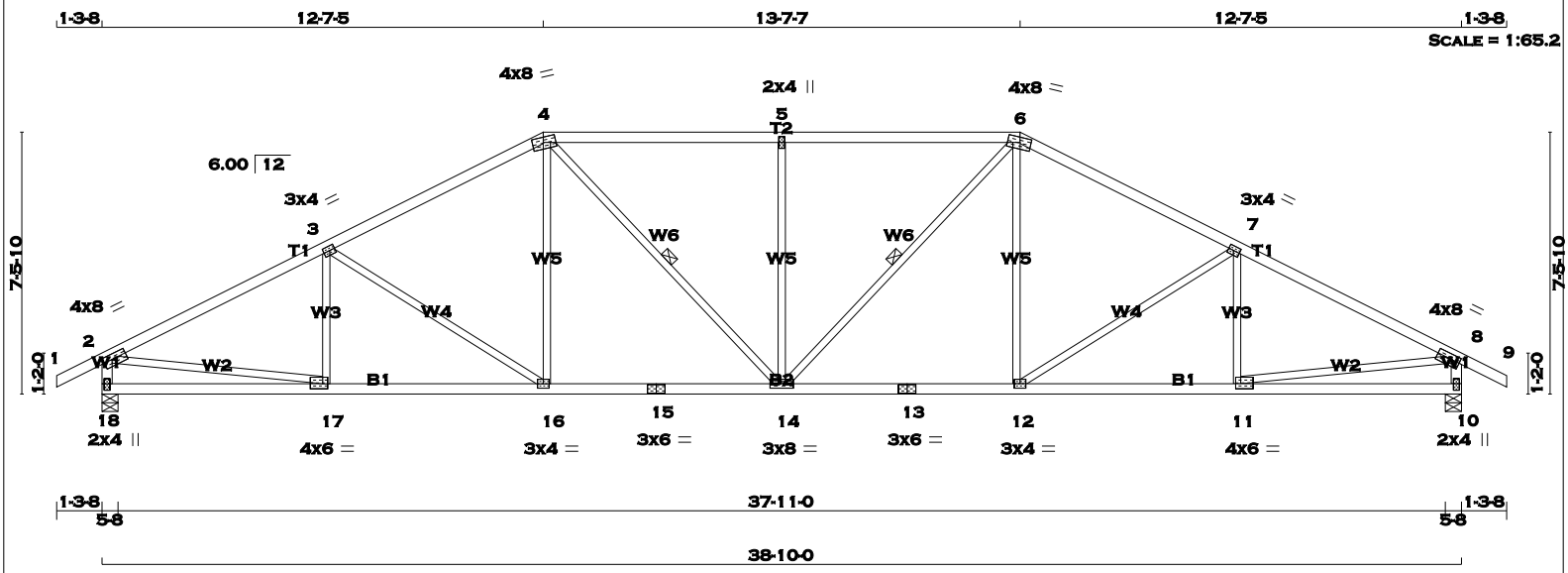
JSI GRIP= 0.90 (7) (INPUT = 0.90)
JSI METAL= 0.65 (16) (INPUT = 1.00)

CONTINUED ON PAGE 2

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



TOTAL WEIGHT = 157 lb

[M][F]

LUMBER

N. L. G. A. RULES

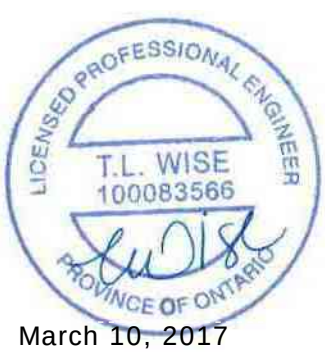
CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 6	2x4	DRY	No.2	SPF
6 - 9	2x4	DRY	No.2	SPF
18 - 2	2x4	DRY	No.2	SPF
10 - 8	2x4	DRY	No.2	SPF
18 - 15	2x4	DRY	No.2	SPF
15 - 13	2x4	DRY	No.2	SPF
13 - 10	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-t	MT20	4.0	8.0	1.75	3.00
3	TMWW-t	MT20	3.0	4.0	1.50	1.75
4	TTWW-m	MT20	4.0	8.0	1.75	3.75
5	TMW+w	MT20	2.0	4.0		
6	TTWW-m	MT20	4.0	8.0	1.75	3.75
7	TMWW-t	MT20	3.0	4.0	1.50	1.75
8	TMVW-t	MT20	4.0	8.0	1.75	3.00
10	BMV1+p	MT20	2.0	4.0	2.25	1.00
11	BMWW-t	MT20	4.0	6.0	1.75	1.75
12	BMWW-t	MT20	3.0	4.0		
13	BS-t	MT20	3.0	6.0		
14	BMWWW-t	MT20	3.0	8.0		
15	BS-t	MT20	3.0	6.0		
16	BMWW-t	MT20	3.0	4.0		
17	BMWW-t	MT20	4.0	6.0	1.75	1.75
18	BMV1+p	MT20	2.0	4.0	2.25	1.00

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.



March 10, 2017

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
18	1947	0	2006	-189	-964	5-8	5-8
10	1947	0	1968	0	-964	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 18 FOR 964 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 10 FOR 964 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 189 LBS FACTORED HORIZONTAL REACTION AT JOINT 18

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
18	1364	967 / 0	0 / 0	0 / 0	149 / -943	396 / 0	0 / 0
10	1364	967 / 0	0 / 0	0 / 0	54 / -943	396 / 0	0 / 0
HORIZONTAL REACTIONS							
18	---	0 / 0	0 / 0	0 / 0	135 / -135	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 18, 10

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 5.66 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 4-14, 6-14. DBS = 20-0-0 . CBF = 43 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
1- 2	0 / 23	-77.3	-77.3	0.10 (1)		10.00
2- 3	-2895 / 1403	-77.3	-77.3	0.65 (7)		3.66
3- 4	-2563 / 1327	-77.3	-77.3	0.64 (7)		3.91
4- 5	-2641 / 1409	-77.3	-77.3	0.62 (7)		3.72
5- 6	-2641 / 1409	-77.3	-77.3	0.62 (7)		3.72
6- 7	-2538 / 1327	-77.3	-77.3	0.64 (8)		3.91
7- 8	-2845 / 1404	-77.3	-77.3	0.65 (8)		3.66
8- 9	0 / 23	-77.3	-77.3	0.10 (1)		10.00
18- 2	-1957 / 994	0.0	0.0	0.19 (1)		6.08
10- 8	-1919 / 994	0.0	0.0	0.19 (1)		6.10
18-17	-171 / 200	-17.5	-17.5	0.16 (11)		6.25
17-16	-1284 / 2648	-17.5	-17.5	0.49 (1)		5.66
16-15	-954 / 2293	-17.5	-17.5	0.44 (1)		6.25
15-14	-95 / 22	-17	-17.5	0.4 (1)		6.25
14-13	-95 / 22	-17	-17.5	0.4 (1)		6.25
13-12	-95 / 22	-17	-17.5	0.4 (1)		6.25
12-11	-10 / 2	-17	-17.5	0.4 (1)		5.99
11-10	-9 / 18	-17.5	-17.5	0.16 (11)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI
17- 3	-233 / 245	0.06 (1)
3-16	-428 / 395	0.47 (3)
16- 4	-140 / 349	0.14 (7)
4-14	-399 / 539	0.27 (8)
14- 5	-681 / 527	0.67 (3)
14- 6	-399 / 571	0.27 (7)
12- 6	-140 / 353	0.14 (8)
12- 7	-437 / 395	0.48 (4)
11- 7	-229 / 245	0.06 (1)
2-17	-1128 / 2624	0.80 (7)
11- 8	-1128 / 2598	0.80 (8)

WIND {40-0 COE WIN {OPE FRO

PRESSURE (20) PSF AT EXTERNAL PEAK STING SYSTEM; INTERNAL MAY BE LOCATED ON LEAST [0-0] FROM SX AWAY

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

KOTT

RECEIVED
TOWN OF MILTON
MAR 20, 2017
17-1078
BUILDING DIVISION

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.29")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (1.29")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.33")

CSI: TC=0.65 (7-8:8) , BC=0.49 (11-12:1) , WB=0.80 (8-11:8) , SSI=0.26 (5-6:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

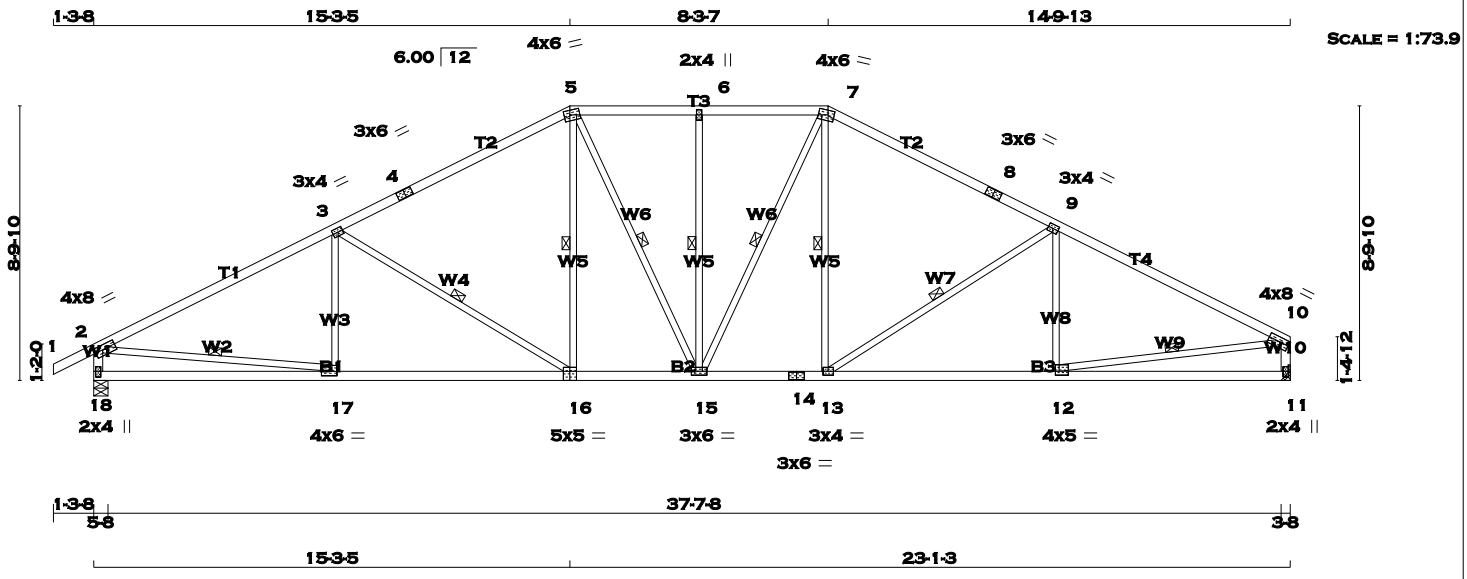
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (4) (INPUT = 0.90)
JSI METAL= 0.65 (13) (INPUT = 1.00)



TOTAL WEIGHT = 163 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 5	2x4	DRY	No.2	SPF
5 - 7	2x4	DRY	No.2	SPF
7 - 8	2x4	DRY	No.2	SPF
8 - 10	2x4	DRY	No.2	SPF
18 - 2	2x4	DRY	No.2	SPF
11 - 10	2x4	DRY	No.2	SPF
18 - 16	2x4	DRY	No.2	SPF
16 - 14	2x4	DRY	No.2	SPF
14 - 11	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

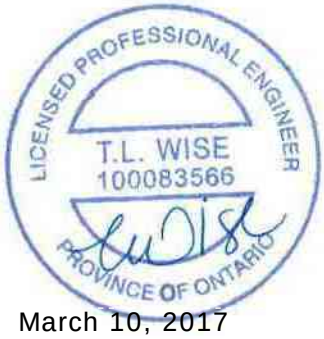
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-t	MT20	4.0	8.0	1.75	3.00
3	TMWW-t	MT20	3.0	4.0	1.50	1.75
4	TS-t	MT20	3.0	6.0		
5	TTWW-m	MT20	4.0	6.0	1.75	2.25
6	TMW+w	MT20	2.0	4.0		
7	TTWW-m	MT20	4.0	6.0	1.75	2.25
8	TS-t	MT20	3.0	6.0		
9	TMWW-t	MT20	3.0	4.0	1.50	1.75
10	TMVW-t	MT20	4.0	8.0	1.75	Edge
11	BMV1+p	MT20	2.0	4.0	2.25	1.00
12	BMWW-t	MT20	4.0	5.0	1.50	1.50
13	BMWW-t	MT20	3.0	4.0		
14	BS-t	MT20	3.0	6.0		
15	BMWWW-t	MT20	3.0	6.0		
16	BSWW-l	MT20	5.0	5.0	Edge	2.50
17	BMWW-t	MT20	4.0	6.0	1.75	2.00
18	BMV1+p	MT20	2.0	4.0	2.25	1.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.



March 10, 2017

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
18	1925	0	1977	236	-923	5-8	5-8
11	1820	0	1852	0	-856	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

PROVIDE ANCHORAGE AT BEARING JOINT 18 FOR 923 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 11 FOR 856 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 236 LBS FACTORED HORIZONTAL REACTION AT JOINT 18

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
18	1349	957 / 0	0 / 0	0 / 0	132 / -912	392 / 0	0 / 0
11	1277	893 / 0	0 / 0	0 / 0	79 / -858	384 / 0	0 / 0
HORIZONTAL REACTIONS							
18	---	0 / 0	0 / 0	0 / 0	169 / -139	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 18

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.28 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 5.67 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 3-16, 5-16, 5-15, 6-15, 7-15, 7-13, 9-13. DBS = 20-0-0 . CBF = 80 LBS.
1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 2-17. DBS = 10-0-0 . CBF = 79 LBS.
1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 10-12. DBS = 12-0-0 . CBF = 91 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 23	-77.3	-77.3	0.10 (1)	10.00	17- 3	-135 / 232	0.05 (3)
2- 3	-2861 / 1336	-77.3	-77.3	0.90 (1)	3.28	3-16	-681 / 554	0.37 (3)
3- 4	-2300 / 1168	-77.3	-77.3	0.84 (7)	3.72	16- 5	-219 / 471	0.11 (7)
4- 5	-2300 / 1168	-77.3	-77.3	0.84 (7)	3.72	5-15	-256 / 233	0.15 (8)
5- 6	-2126 / 1158	-77.3	-77.3	0.31 (7)	4.54	15- 6	-405 / 315	0.19 (3)
6- 7	-2126 / 1158	-77.3	-77.3	0.31 (7)	4.54	15- 7	-278 / 299	0.17 (7)
7- 8	-2260 / 1154	-77.3	-77.3	0.80 (8)	3.82	13- 7	-199 / 425	0.10 (8)
8- 9	-2260 / 1154	-77.3	-77.3	0.80 (8)	3.82	13- 9	-577 / 494	0.30 (4)
9-10	-2684 / 1268	-77.3	-77.3	0.81 (1)	3.46	12- 9	-205 / 260	0.08 (1)
18- 2	-1921 / 960	0.0	0.0	0.19 (1)	6.13	2-17	-1047 / 2591	0.57 (1)
11-10	-1797 / 892	0.0	0.0	0.18 (1)	6.28	12-10	-1025 / 2453	0.54 (1)
18-17	-219 / 193	-17.5	-17.5	0.25 (11)	6.25			
17-16	-1260 / 2624	-17.5	-17.5	0.53 (1)	5.67			
16-15	-792 / 2051	-17.5	-17.5	0.40 (1)	6.25			
15-14	-683 / 2008	-17.5	-17.5	0.39 (1)	6.25			
14-13	-683 / 2008	-17.5	-17.5	0.39 (1)	6.25			
13-12	-993 / 2428	-17.5	-17.5	0.50 (1)	6.20			
12-11	-11 / 23	-17.5	-17.5	0.24 (11)	6.25			

RECEIVED
TOWN OF MILTON
MAR 29, 2017
17-4978
BUILDING DIVISION

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.28")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (1.28")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.30")

CSI: TC=0.90 (2-3:1) , BC=0.53 (16-17:1) , WB=0.57 (2-17:1) , SSI=0.26 (2-3:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

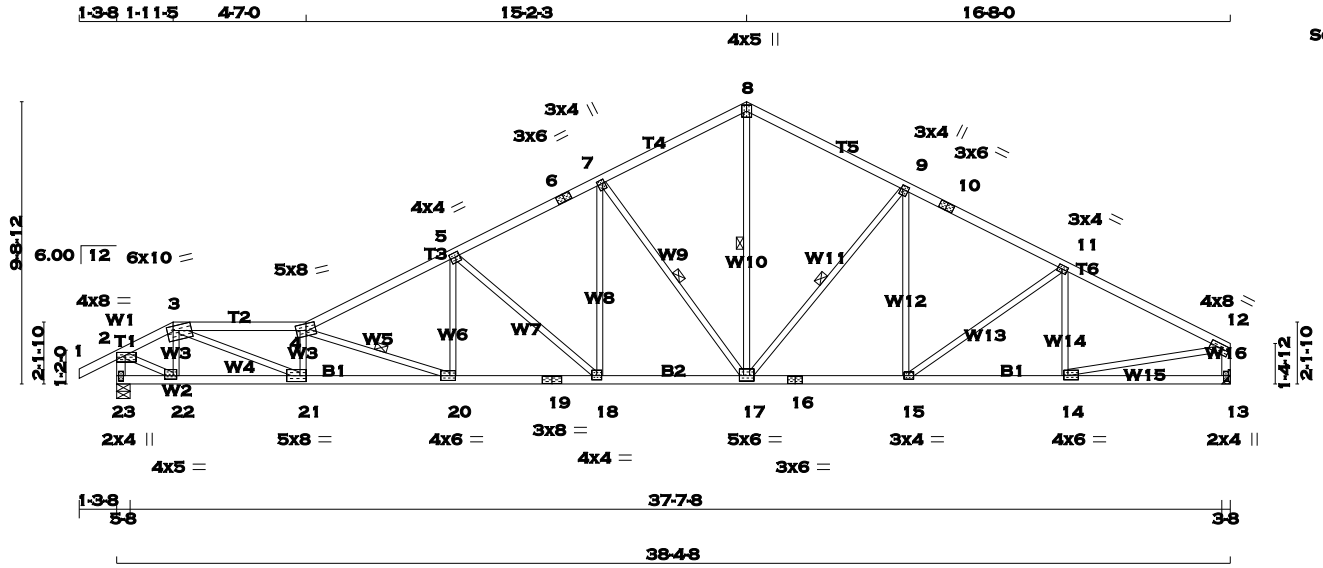
JSI GRIP= 0.90 (12) (INPUT = 0.90)
JSI METAL= 0.73 (12) (INPUT = 1.00)

CONTINUED ON PAGE 2

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



TOTAL WEIGHT = 161 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 3	2x4	DRY	No.2	SPF
3 - 4	2x4	DRY	No.2	SPF
4 - 6	2x4	DRY	No.2	SPF
6 - 8	2x4	DRY	No.2	SPF
8 - 10	2x4	DRY	No.2	SPF
10 - 12	2x4	DRY	No.2	SPF
23 - 2	2x4	DRY	No.2	SPF
13 - 12	2x4	DRY	No.2	SPF
23 - 19	2x4	DRY	No.2	SPF
19 - 16	2x4	DRY	No.2	SPF
16 - 13	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

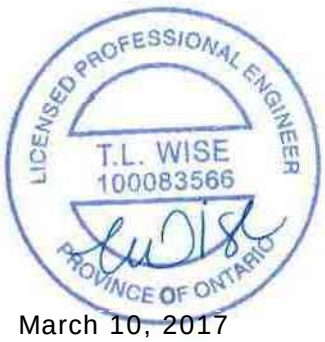
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-p	MT20	4.0	8.0	1.25	3.50
3	TTWW-m	MT20	6.0	10.0	1.75	2.50
4	TTWW-m	MT20	5.0	8.0	2.75	4.00
5	TMWW-t	MT20	4.0	4.0	1.75	1.50
6	TS-t	MT20	3.0	6.0		
7	TMWW+t	MT20	3.0	4.0	1.50	0.75
8	TTW+p	MT20	4.0	5.0		
9	TMWW+t	MT20	3.0	4.0	1.75	0.75
10	TS-t	MT20	3.0	6.0		
11	TMWW-t	MT20	3.0	4.0	1.50	1.75
12	TMVW-t	MT20	4.0	8.0	1.75	Edge
13	BMV1+p	MT20	2.0	4.0	2.25	1.00
14	BMWW-t	MT20	4.0	6.0	1.75	1.75
15	BMWW-t	MT20	3.0	4.0		
16	BS-t	MT20	3.0	6.0		
17	BMWWW-t	MT20	5.0	6.0	2.25	3.00
18	BMWW-t	MT20	4.0	4.0	1.75	2.00
19	BS-t	MT20	3.0	8.0		
20	BMWW-t	MT20	4.0	6.0	2.00	2.25
21	BMWW-t	MT20	5.0	8.0	2.50	2.50
22	BMWW-t	MT20	4.0	5.0	1.50	1.50
23	BMV1+p	MT20	2.0	4.0	2.25	1.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.



March 10, 2017

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
23	1925	0	1972	260	-898	5-8	5-8
13	1820	0	1856	0	-770	HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-8	

PROVIDE ANCHORAGE AT BEARING JOINT 23 FOR 898 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 13 FOR 770 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 260 LBS FACTORED HORIZONTAL REACTION AT JOINT 23

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
23	1349	957 / 0	0 / 0	0 / 0	118 / -893	392 / 0	0 / 0
13	1277	893 / 0	0 / 0	0 / 0	92 / -797	384 / 0	0 / 0
HORIZONTAL REACTIONS							
23	---	0 / 0	0 / 0	0 / 0	186 / -152	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 23

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.68 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 4.30 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 4-20. DBS = 6-0-0 . CBF = 75 LBS.
1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 7-17. DBS = 12-0-0 . CBF = 88 LBS.
1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 8-17, 9-17. DBS = 20-0-0 . CBF = 93 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 23	-77.3	-77.3	0.10 (1)	10.00	22- 3	-688 / 354	0.10 (1)
2- 3	-2200 / 1003	-77.3	-77.3	0.18 (7)	4.60	3-21	-1583 / 3722	0.82 (1)
3- 4	-5371 / 2374	-77.3	-77.3	0.56 (1)	2.68	21- 4	-1400 / 696	0.20 (1)
4- 5	-3905 / 1719	-77.3	-77.3	0.47 (7)	3.30	4-20	-2069 / 1005	0.46 (1)
5- 6	-2875 / 1324	-77.3	-77.3	0.42 (7)	3.91	20- 5	-293 / 832	0.18 (1)
6- 7	-2875 / 1324	-77.3	-77.3	0.42 (7)	3.91	5-18	-1253 / 715	1.00 (3)
7- 8	-2094 / 1029	-77.3	-77.3	0.38 (7)	4.47	18- 7	-398 / 892	0.36 (7)
8- 9	-2102 / 1055	-77.3	-77.3	0.41 (8)	4.42	7-17	-1224 / 763	0.59 (3)
9-10	-2496 / 1098	-77.3	-77.3	0.44 (8)	4.11	17- 8	-724 / 1525	0.44 (7)
10-11	-2496 / 1098	-77.3	-77.3	0.44 (8)	4.11	17- 9	-625 / 506	0.31 (4)
11-12	-2613 / 1087	-77.3	-77.3	0.44 (8)	3.97	15- 9	-75 / 188	0.06 (8)
23- 2	-1992 / 916	0.0	0.0	0.20 (1)	6.03	15-11	-164 / 244	0.14 (4)
13-12	-1813 / 797	0.0	0.0	0.18 (1)	6.26	14-11	-342 / 249	0.09 (1)
						2-22	-878 / 2122	0.47 (1)
23-22	-243 / 217	-17.5	-17.5	0.10 (1)	6.25	14-12	-847 / 2381	0.53 (1)
22-21	-1033 / 1971	-17.5	-17.5	0.43 (1)	6.12			
21-20	-2538 / 5494	-17.5	-17.5	0.99 (1)	4.30			
20-19	-1593 / 3550	-17.5	-17.5	0.62 (1)	5.24			
19-18	-1593 / 3550	-17.5	-17.5	0.62 (1)	5.24			
18-17	-1051 / 2601	-17.5	-17.5	0.47 (1)	6.13			
17-16	-742 / 2234	-17.5	-17.5	0.42 (1)	6.25			
16-15	-742 / 2234	-17.5	-17.5	0.42 (1)	6.25			
15-14	-850 / 2333	-17.5	-17.5	0.42 (1)	6.25			
14-13	-11 / 23	-17.5	-17.5	0.12 (11)	6.25			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.28")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.35")
ALLOWABLE DEFL.(TL)= L/360 (1.28")
CALCULATED VERT. DEFL.(TL) = L/ 749 (0.61")

CSI: TC=0.56 (3-4:1) , BC=0.99 (20-21:1) , WB=1.00 (5-18:3) , SSI=0.18 (11-12:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

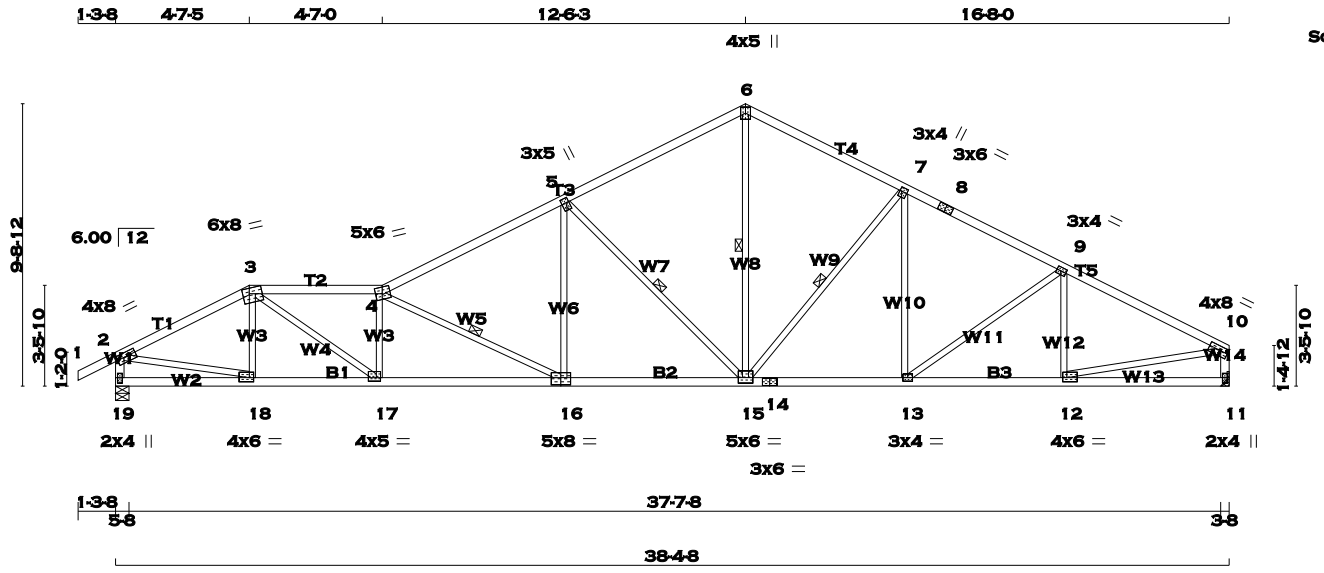
JSI GRIP= 0.90 (18) (INPUT = 0.90)
JSI METAL= 0.92 (19) (INPUT = 1.00)

CONTINUED ON PAGE 2

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



TOTAL WEIGHT = 158 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 3	2x4	DRY	No.2	SPF
3 - 4	2x4	DRY	No.2	SPF
4 - 6	2x4	DRY	No.2	SPF
6 - 8	2x4	DRY	No.2	SPF
8 - 10	2x4	DRY	No.2	SPF
19 - 2	2x4	DRY	No.2	SPF
11 - 10	2x4	DRY	No.2	SPF
19 - 16	2x4	DRY	No.2	SPF
16 - 14	2x4	DRY	No.2	SPF
14 - 11	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

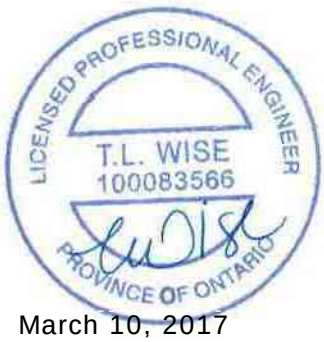
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-t	MT20	4.0	8.0	1.75	3.00
3	TTWW-m	MT20	6.0	8.0	2.25	2.75
4	TTWW-m	MT20	5.0	6.0	2.75	2.75
5	TMWW+t	MT20	3.0	5.0	2.00	0.75
6	TTW+p	MT20	4.0	5.0		
7	TMWW+t	MT20	3.0	4.0	1.75	0.75
8	TS-t	MT20	3.0	6.0		
9	TMWW-t	MT20	3.0	4.0	1.50	1.75
10	TMVW-t	MT20	4.0	8.0	1.75	Edge
11	BMV1+p	MT20	2.0	4.0	2.25	1.00
12	BMWW-t	MT20	4.0	6.0	1.75	1.75
13	BMWW-t	MT20	3.0	4.0		
14	BS-t	MT20	3.0	6.0		
15	BMWWW-t	MT20	5.0	6.0	2.25	3.00
16	BSWW-l	MT20	5.0	8.0	3.00	4.00
17	BMWW-t	MT20	4.0	5.0	1.50	1.75
18	BMWW-t	MT20	4.0	6.0	1.75	1.75
19	BMV1+p	MT20	2.0	4.0	2.25	1.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.



March 10, 2017

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
19	1925	0	1972	260	-898	5-8	5-8
11	1820	0	1856	0	-770	HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-8	

PROVIDE ANCHORAGE AT BEARING JOINT 19 FOR 898 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 11 FOR 770 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 260 LBS FACTORED HORIZONTAL REACTION AT JOINT 19

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
19	1349	957 / 0	0 / 0	0 / 0	118 / -893	392 / 0	0 / 0
11	1277	893 / 0	0 / 0	0 / 0	92 / -797	384 / 0	0 / 0
HORIZONTAL REACTIONS							
19	---	0 / 0	0 / 0	0 / 0	186 / -152	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 19

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.23 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 4.89 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 4-16, 5-15. DBS = 10-0-0 . CBF = 89 LBS.
1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 6-15, 7-15. DBS = 20-0-0 . CBF = 92 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
1- 2	0 / 23	-77.3	-77.3	0.10 (1)	10.00
2- 3	-2700 / 1207	-77.3	-77.3	0.42 (7)	4.02
3- 4	-4075 / 1833	-77.3	-77.3	0.45 (7)	3.23
4- 5	-3097 / 1385	-77.3	-77.3	0.65 (7)	3.52
5- 6	-2103 / 1015	-77.3	-77.3	0.59 (7)	4.24
6- 7	-2102 / 1054	-77.3	-77.3	0.41 (8)	4.42
7- 8	-2496 / 1099	-77.3	-77.3	0.44 (8)	4.11
8- 9	-2496 / 1099	-77.3	-77.3	0.44 (8)	4.11
9-10	-2613 / 1087	-77.3	-77.3	0.44 (8)	3.97
19- 2	-1937 / 922	0.0	0.0	0.19 (1)	6.10
11-10	-1813 / 797	0.0	0.0	0.18 (1)	6.26
19-18	-243 / 217	-17.5	-17.5	0.08 (11)	6.25
18-17	-1184 / 2445	-17.5	-17.5	0.42 (1)	5.85
17-16	-1885 / 4150	-17.5	-17.5	0.73 (1)	4.89
16-15	-1191 / 2828	-17.5	-17.5	0.52 (1)	5.84
15-14	-742 / 2234	-17.5	-17.5	0.43 (1)	6.25
14-13	-742 / 2234	-17.5	-17.5	0.43 (1)	6.25
13-12	-850 / 2333	-17.5	-17.5	0.42 (1)	6.25
12-11	-11 / 23	-17.5	-17.5	0.12 (11)	6.25

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
18- 3	-344 / 230	0.07 (1)
3-17	-837 / 2045	0.45 (1)
17- 4	-1104 / 558	0.22 (1)
4-16	-1481 / 777	0.49 (1)
16- 5	-270 / 790	0.19 (7)
5-15	-1350 / 840	0.71 (3)
15- 6	-698 / 1499	0.43 (7)
15- 7	-624 / 508	0.31 (4)
13- 7	-77 / 187	0.06 (8)
13- 9	-165 / 244	0.14 (4)
12- 9	-341 / 250	0.09 (1)
2-18	-963 / 2442	0.54 (1)
12-10	-887 / 2381	0.53 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.28")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.26")
ALLOWABLE DEFL.(TL)= L/360 (1.28")
CALCULATED VERT. DEFL.(TL) = L/ 964 (0.48")

CSI: TC=0.65 (4-5:7) , BC=0.73 (16-17:1) , WB=0.71 (5-15:3) , SSI=0.21 (4-5:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

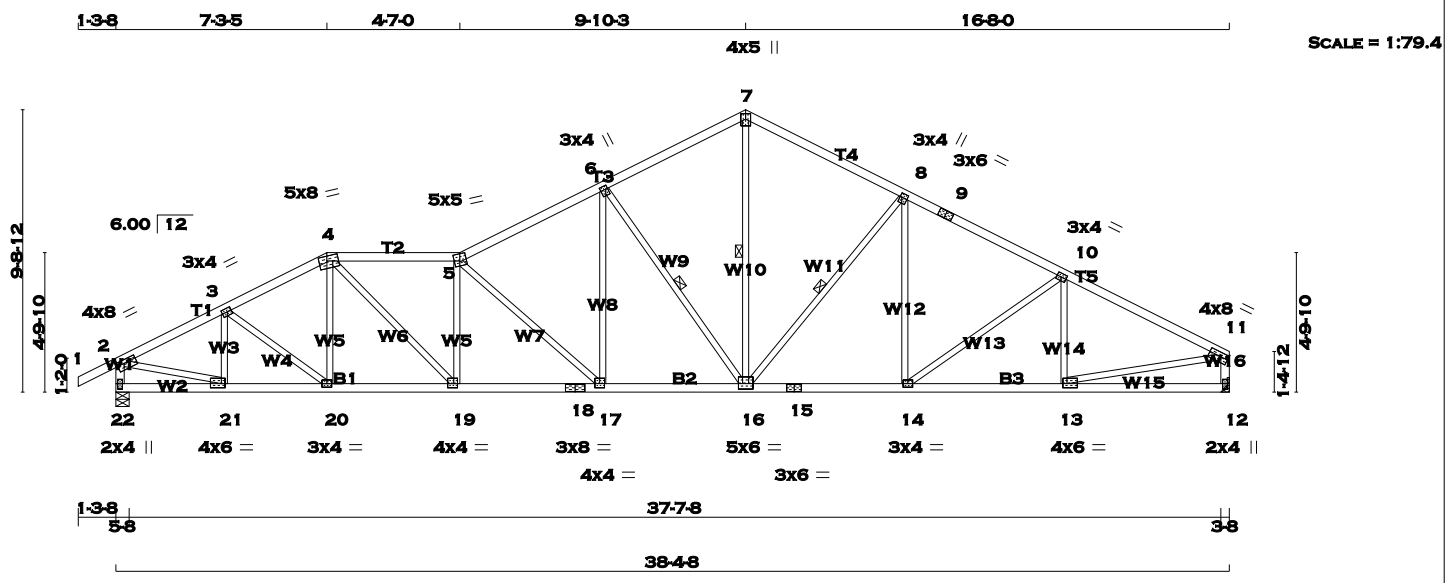
JSI GRIP= 0.89 (10) (INPUT = 0.90)
JSI METAL= 0.84 (16) (INPUT = 1.00)

CONTINUED ON PAGE 2

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



TOTAL WEIGHT = 165 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 5	2x4	DRY	No.2	SPF
5 - 7	2x4	DRY	No.2	SPF
7 - 9	2x4	DRY	No.2	SPF
9 - 11	2x4	DRY	No.2	SPF
22 - 2	2x4	DRY	No.2	SPF
12 - 11	2x4	DRY	No.2	SPF
22 - 18	2x4	DRY	No.2	SPF
18 - 15	2x4	DRY	No.2	SPF
15 - 12	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

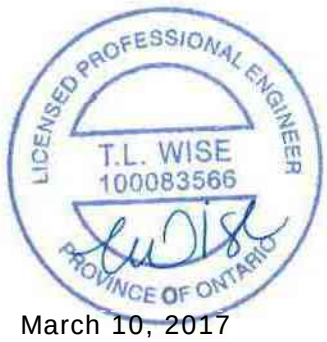
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-t	MT20	4.0	8.0	1.50	3.00
3	TMWW-t	MT20	3.0	4.0	1.50	1.75
4	TTWW-m	MT20	5.0	8.0	2.00	3.25
5	TTWW-m	MT20	5.0	5.0	3.00	2.25
6	TMWW+t	MT20	3.0	4.0	1.50	0.75
7	TTW+p	MT20	4.0	5.0	2.25	2.00
8	TMWW+t	MT20	3.0	4.0	1.75	0.75
9	TS-t	MT20	3.0	6.0		
10	TMWW-t	MT20	3.0	4.0	1.50	1.75
11	TMVW-t	MT20	4.0	8.0	1.75	Edge
12	BMV1+p	MT20	2.0	4.0	2.25	1.00
13	BMWW-t	MT20	4.0	6.0	1.75	1.75
14	BMWW-t	MT20	3.0	4.0		
15	BS-t	MT20	3.0	6.0		
16	BMWWW-t	MT20	5.0	6.0	2.25	3.00
17	BMWW-t	MT20	4.0	4.0	1.75	2.00
18	BS-t	MT20	3.0	8.0		
19	BMWW-t	MT20	4.0	4.0	1.75	1.50
20	BMWW-t	MT20	3.0	4.0		
21	BMWW-t	MT20	4.0	6.0	1.75	1.50
22	BMV1+p	MT20	2.0	4.0	2.25	1.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.



March 10, 2017

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
22	1925	0	1972	260	-898	5-8	5-8
12	1820	0	1856	0	-770	HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-8	

PROVIDE ANCHORAGE AT BEARING JOINT 22 FOR 898 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 12 FOR 770 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 260 LBS FACTORED HORIZONTAL REACTION AT JOINT 22

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
22	1349	957 / 0	0 / 0	0 / 0	118 / -893	392 / 0	0 / 0
12	1277	893 / 0	0 / 0	0 / 0	92 / -797	384 / 0	0 / 0
HORIZONTAL REACTIONS							
22	---	0 / 0	0 / 0	0 / 0	186 / -152	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 22

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.59 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 5.35 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 6-16. DBS = 12-0-0 . CBF = 88 LBS.
1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 7-16. DBS = 18-0-0 . CBF = 85 LBS.
1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 8-16. DBS = 20-0-0 . CBF = 73 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 23	-77.3	-77.3	0.10 (1)	10.00	21- 3	-487 / 284	0.09 (1)
2- 3	-2613 / 1196	-77.3	-77.3	0.30 (7)	4.19	3-20	-9 / 142	0.03 (1)
3- 4	-2760 / 1299	-77.3	-77.3	0.32 (7)	4.10	20- 4	-6 / 53	0.02 (11)
4- 5	-3373 / 1556	-77.3	-77.3	0.41 (7)	3.59	4-19	-493 / 1297	0.33 (7)
5- 6	-2836 / 1303	-77.3	-77.3	0.46 (7)	3.89	19- 5	-845 / 416	0.28 (1)
6- 7	-2097 / 1036	-77.3	-77.3	0.42 (7)	4.46	5-17	-1139 / 630	0.92 (3)
7- 8	-2102 / 1055	-77.3	-77.3	0.41 (8)	4.42	17- 6	-362 / 853	0.33 (7)
8- 9	-2496 / 1098	-77.3	-77.3	0.44 (8)	4.11	6-16	-1228 / 769	0.59 (3)
9-10	-2496 / 1098	-77.3	-77.3	0.44 (8)	4.11	16- 7	-738 / 1538	0.45 (7)
10-11	-2613 / 1087	-77.3	-77.3	0.44 (8)	3.97	16- 8	-625 / 506	0.31 (4)
22- 2	-1936 / 912	0.0	0.0	0.19 (1)	6.10	14- 8	-75 / 188	0.06 (8)
12-11	-1813 / 797	0.0	0.0	0.18 (1)	6.26	14-10	-164 / 244	0.14 (4)
						13-10	-342 / 249	0.09 (1)
22-21	-243 / 217	-17.5	-17.5	0.06 (8)	6.25	2-21	-991 / 2400	0.53 (1)
21-20	-1209 / 2393	-17.5	-17.5	0.41 (1)	5.82	13-11	-887 / 2381	0.53 (1)
20-19	-1151 / 2492	-17.5	-17.5	0.43 (1)	5.93			
19-18	-1509 / 3426	-17.5	-17.5	0.59 (1)	5.35			
18-17	-1509 / 3426	-17.5	-17.5	0.59 (1)	5.35			
17-16	-1044 / 2585	-17.5	-17.5	0.47 (1)	6.15			
16-15	-742 / 2234	-17.5	-17.5	0.42 (1)	6.25			
15-14	-742 / 2234	-17.5	-17.5	0.42 (1)	6.25			
14-13	-850 / 2333	-17.5	-17.5	0.42 (1)	6.25			
13-12	-11 / 23	-17.5	-17.5	0.12 (11)	6.25			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.28")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.22")
ALLOWABLE DEFL.(TL)= L/360 (1.28")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.39")

CSI: TC=0.46 (5-6:7) , BC=0.59 (17-19:1) , WB=0.92 (5-17:3) , SSI=0.18 (10-11:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

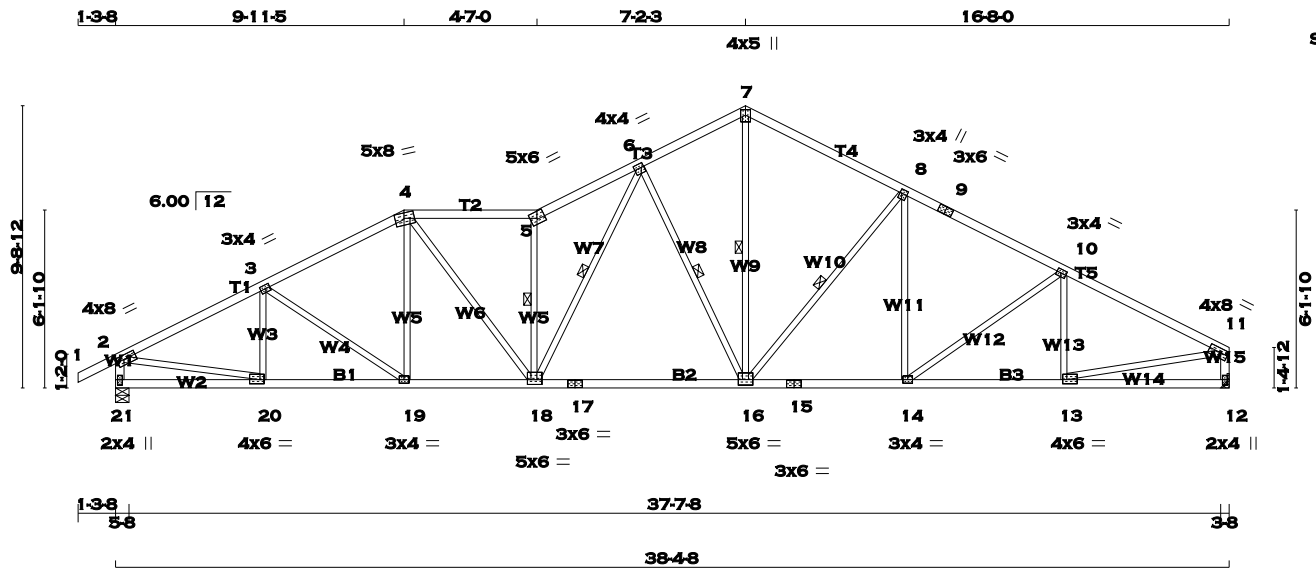
JSI GRIP= 0.90 (4) (INPUT = 0.90)
JSI METAL= 0.88 (18) (INPUT = 1.00)

CONTINUED ON PAGE 2

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



TOTAL WEIGHT = 167 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 5	2x4	DRY	No.2	SPF
5 - 7	2x4	DRY	No.2	SPF
7 - 9	2x4	DRY	No.2	SPF
9 - 11	2x4	DRY	No.2	SPF
21 - 2	2x4	DRY	No.2	SPF
12 - 11	2x4	DRY	No.2	SPF
21 - 17	2x4	DRY	No.2	SPF
17 - 15	2x4	DRY	No.2	SPF
15 - 12	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

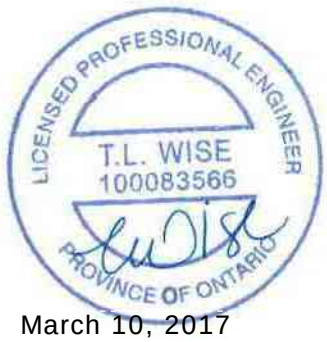
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-t	MT20	4.0	8.0	1.75	3.00
3	TMWW-t	MT20	3.0	4.0	1.50	1.75
4	TTWW-m	MT20	5.0	8.0	2.25	3.75
5	TTW-h	MT20	5.0	6.0	2.75	3.25
6	TMWW-t	MT20	4.0	4.0	1.75	1.00
7	TTW+p	MT20	4.0	5.0	2.25	2.00
8	TMWW+t	MT20	3.0	4.0	1.75	0.75
9	TS-t	MT20	3.0	6.0		
10	TMWW-t	MT20	3.0	4.0	1.50	1.75
11	TMVW-t	MT20	4.0	8.0	1.75	Edge
12	BMV1+p	MT20	2.0	4.0	2.25	1.00
13	BMWW-t	MT20	4.0	6.0	1.75	1.75
14	BMWW-t	MT20	3.0	4.0		
15	BS-t	MT20	3.0	6.0		
16	BMWWW-t	MT20	5.0	6.0	2.25	3.00
17	BS-t	MT20	3.0	6.0		
18	BMWWW-t	MT20	5.0	6.0	2.00	1.50
19	BMWW-t	MT20	3.0	4.0		
20	BMWW-t	MT20	4.0	6.0	1.75	1.75
21	BMV1+p	MT20	2.0	4.0	2.25	1.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.



March 10, 2017

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
21	1925	0	1972	260	-898	5-8	5-8
12	1820	0	1856	0	-770	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

PROVIDE ANCHORAGE AT BEARING JOINT 21 FOR 898 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 12 FOR 770 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 260 LBS FACTORED HORIZONTAL REACTION AT JOINT 21

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
21	1349	957 / 0	0 / 0	0 / 0	118 / -893	392 / 0	0 / 0
12	1277	893 / 0	0 / 0	0 / 0	92 / -797	384 / 0	0 / 0
HORIZONTAL REACTIONS							
21	---	0 / 0	0 / 0	0 / 0	186 / -152	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 21

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 5.73 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 5-18. DBS = 8-0-0 . CBF = 85 LBS.
1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 6-18, 8-16. DBS = 20-0-0 . CBF = 79 LBS.
1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 6-16. DBS = 12-0-0 . CBF = 81 LBS.
1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 7-16. DBS = 18-0-0 . CBF = 87 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
1- 2	0 / 23	-77.3	-77.3	0.10 (1)	10.00
2- 3	-2770 / 1260	-77.3	-77.3	0.45 (7)	3.95
3- 4	-2660 / 1266	-77.3	-77.3	0.45 (7)	4.03
4- 5	-2875 / 1359	-77.3	-77.3	0.38 (7)	3.89
5- 6	-3234 / 1589	-77.3	-77.3	0.37 (7)	3.77
6- 7	-2097 / 1053	-77.3	-77.3	0.29 (7)	4.58
7- 8	-2105 / 1053	-77.3	-77.3	0.41 (8)	4.42
8- 9	-2495 / 1099	-77.3	-77.3	0.44 (8)	4.11
9-10	-2495 / 1099	-77.3	-77.3	0.44 (8)	4.11
10-11	-2614 / 1087	-77.3	-77.3	0.44 (8)	3.97
21- 2	-1931 / 920	0.0	0.0	0.19 (1)	6.11
12-11	-1813 / 796	0.0	0.0	0.18 (1)	6.26
21-20	-243 / 217	-17.5	-17.5	0.10 (11)	6.25
20-19	-1250 / 2538	-17.5	-17.5	0.43 (1)	5.73
19-18	-1050 / 2395	-17.5	-17.5	0.41 (1)	6.11
18-17	-919 / 2362	-17.5	-17.5	0.48 (1)	6.25
17-16	-919 / 2362	-17.5	-17.5	0.48 (1)	6.25
16-15	-743 / 2233	-17.5	-17.5	0.46 (1)	6.25
15-14	-743 / 2233	-17.5	-17.5	0.46 (1)	6.25
14-13	-850 / 2334	-17.5	-17.5	0.42 (1)	6.25
13-12	-11 / 23	-17.5	-17.5	0.13 (11)	6.25

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
20- 3	-332 / 252	0.07 (1)
3-19	-177 / 245	0.11 (3)
19- 4	-99 / 185	0.06 (7)
4-18	-352 / 852	0.35 (8)
18- 5	-1746 / 936	0.42 (1)
18- 6	-709 / 1311	0.33 (7)
6-16	-1133 / 753	0.53 (3)
16- 7	-777 / 1584	0.47 (7)
16- 8	-618 / 510	0.30 (4)
14- 8	-80 / 180	0.07 (8)
14-10	-166 / 243	0.14 (4)
13-10	-340 / 250	0.09 (1)
2-20	-1021 / 2520	0.55 (1)
13-11	-847 / 2382	0.53 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.28")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.22")
ALLOWABLE DEFL.(TL)= L/360 (1.28")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.41")

CSI: TC=0.45 (3-4:7) , BC=0.48 (16-18:1) , WB=0.55 (2-20:1) , SSI=0.18 (10-11:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

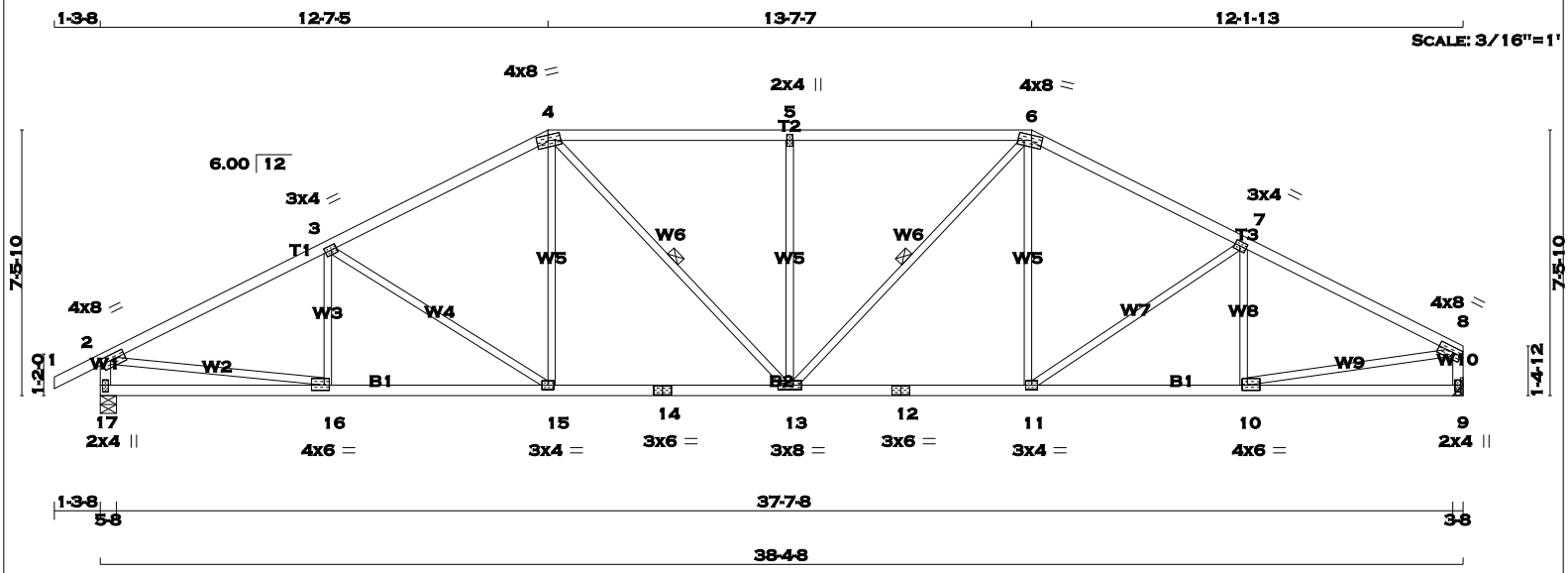
JSI GRIP= 0.90 (2) (INPUT = 0.90)
JSI METAL= 0.66 (17) (INPUT = 1.00)

CONTINUED ON PAGE 2

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



TOTAL WEIGHT = 155 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 6	2x4	DRY	No.2	SPF
6 - 8	2x4	DRY	No.2	SPF
17 - 2	2x4	DRY	No.2	SPF
9 - 8	2x4	DRY	No.2	SPF
17 - 14	2x4	DRY	No.2	SPF
14 - 12	2x4	DRY	No.2	SPF
12 - 9	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

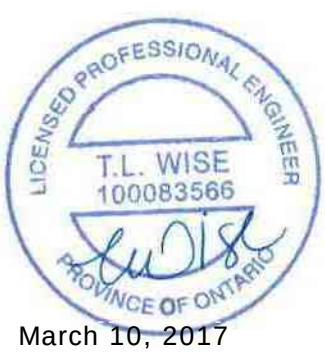
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-t	MT20	4.0	8.0	1.75	3.00
3	TMWW-t	MT20	3.0	4.0	1.50	1.75
4	TTWW-m	MT20	4.0	8.0	1.75	3.75
5	TMW+w	MT20	2.0	4.0		
6	TTWW-m	MT20	4.0	8.0	1.75	3.50
7	TMWW-t	MT20	3.0	4.0	1.50	1.75
8	TMVW-t	MT20	4.0	8.0	1.75	Edge
9	BMV1+p	MT20	2.0	4.0	2.25	1.00
10	BMWW-t	MT20	4.0	6.0	1.75	1.75
11	BMWW-t	MT20	3.0	4.0		
12	BS-t	MT20	3.0	6.0		
13	BMWWW-t	MT20	3.0	8.0		
14	BS-t	MT20	3.0	6.0		
15	BMWW-t	MT20	3.0	4.0		
16	BMWW-t	MT20	4.0	6.0	1.75	1.75
17	BMV1+p	MT20	2.0	4.0	2.25	1.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.



March 10, 2017

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
17	1925	0	1984	206	-955	IN-SX 5-8	IN-SX 5-8
9	1820	0	1844	0	-889	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

PROVIDE ANCHORAGE AT BEARING JOINT 17 FOR 955 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 9 FOR 889 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 206 LBS FACTORED HORIZONTAL REACTION AT JOINT 17

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
17	1349	957 / 0	0 / 0	0 / 0	148 / -934	392 / 0	0 / 0
9	1277	893 / 0	0 / 0	0 / 0	61 / -882	384 / 0	0 / 0
HORIZONTAL REACTIONS							
17	---	0 / 0	0 / 0	0 / 0	147 / -121	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 17

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.69 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 5.65 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 4-13, 6-13. DBS = 20-0-0 . CBF = 45 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FROM	FACTORED VERT. LOAD LC1 (PLF) TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 23	-77.3	-77.3	0.10 (1)	10.00	16- 3	-228 / 243	0.06 (1)
2- 3	-2856 / 1388	-77.3	-77.3	0.65 (7)	3.69	3-15	-433 / 397	0.48 (3)
3- 4	-2520 / 1310	-77.3	-77.3	0.64 (7)	3.94	15- 4	-141 / 351	0.14 (7)
4- 5	-2582 / 1385	-77.3	-77.3	0.61 (7)	3.77	4-13	-383 / 509	0.26 (8)
5- 6	-2582 / 1385	-77.3	-77.3	0.61 (7)	3.77	13- 5	-682 / 527	0.67 (3)
6- 7	-2448 / 1284	-77.3	-77.3	0.60 (8)	4.02	13- 6	-411 / 602	0.28 (7)
7- 8	-2648 / 1305	-77.3	-77.3	0.60 (8)	3.85	11- 6	-115 / 299	0.11 (8)
17- 2	-1935 / 985	0.0	0.0	0.19 (1)	6.11	11- 7	-325 / 331	0.34 (4)
9- 8	-1797 / 918	0.0	0.0	0.18 (1)	6.27	10- 7	-309 / 278	0.09 (1)
						2-16	-1114 / 2590	0.79 (7)
17-16	-188 / 160	-17.5	-17.5	0.16 (11)	6.25	10- 8	-1080 / 2429	0.71 (8)
16-15	-1288 / 2608	-17.5	-17.5	0.48 (1)	5.65			
15-14	-956 / 2249	-17.5	-17.5	0.43 (1)	6.25			
14-13	-956 / 2	-17.5	-17.5	0.43 (1)	6.25			
13-12	-956 / 2	-17.5	-17.5	0.43 (1)	6.25			
12-11	-956 / 2	-17.5	-17.5	0.43 (1)	6.25			
11-10	-1041 / 2	-17.5	-17.5	0.47 (1)	6.12			
10- 9	-111 / 29	-17.5	-17.5	0.14 (11)	6.25			

WIND {40-0 COE WIN {OPE FRO PRESSURE (20) PSF AT EXTERNAL PEAK STING SYSTEM; INTERNAL MAY BE LOCATED ON LEAST {4-0} FROM SX AWAY

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.28")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (1.28")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.31")

CSI: TC=0.65 (2-3:7) , BC=0.48 (15-16:1) , WB=0.79 (2-16:7) , SSI=0.26 (4-5:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

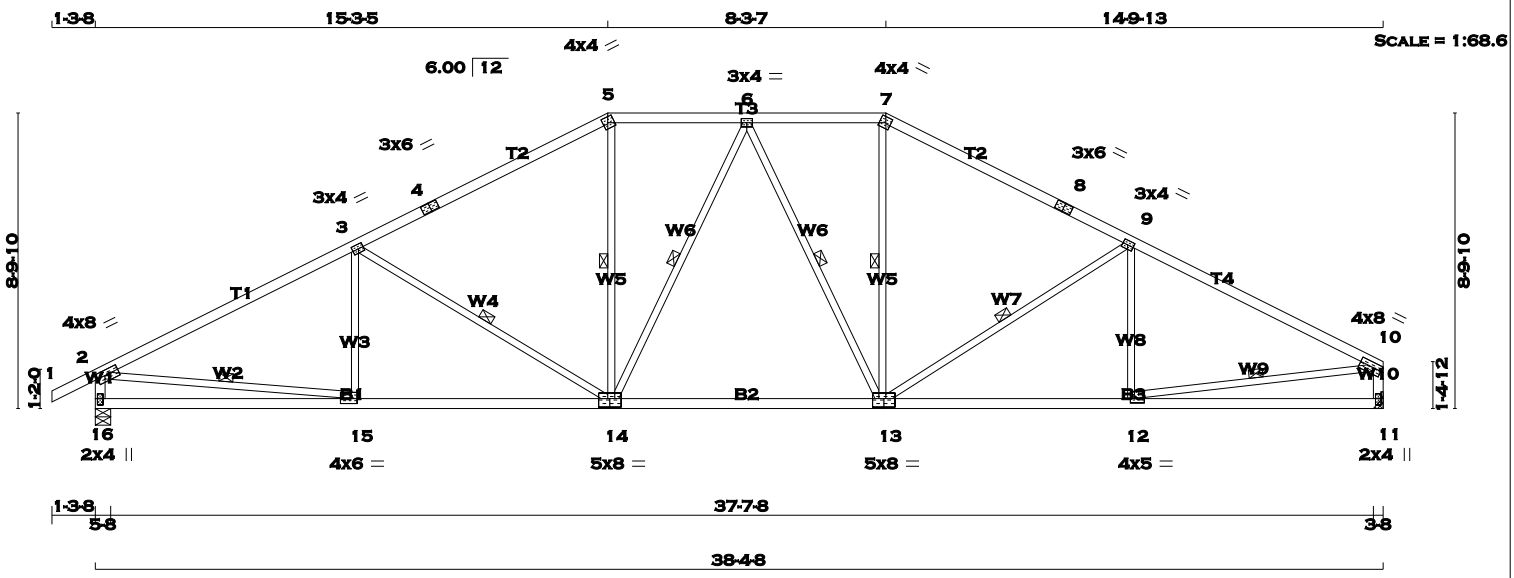
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (8) (INPUT = 0.90)
JSI METAL= 0.64 (14) (INPUT = 1.00)



TOTAL WEIGHT = 157 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 5	2x4	DRY	No.2	SPF
5 - 7	2x4	DRY	No.2	SPF
7 - 8	2x4	DRY	No.2	SPF
8 - 10	2x4	DRY	No.2	SPF
16 - 2	2x4	DRY	No.2	SPF
11 - 10	2x4	DRY	No.2	SPF
16 - 14	2x4	DRY	No.2	SPF
14 - 13	2x4	DRY	No.2	SPF
13 - 11	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

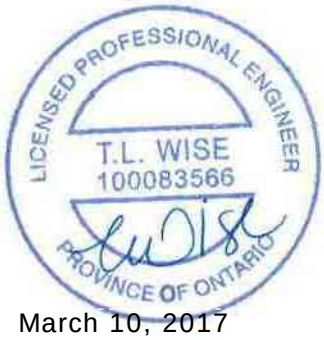
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-t	MT20	4.0	8.0	1.75	3.00
3	TMWW-t	MT20	3.0	4.0	1.50	1.75
4	TS-t	MT20	3.0	6.0		
5	TTW-h	MT20	4.0	4.0	2.00	1.75
6	TMWW-t	MT20	3.0	4.0		
7	TTW-h	MT20	4.0	4.0	2.00	1.75
8	TS-t	MT20	3.0	6.0		
9	TMWW-t	MT20	3.0	4.0	1.50	1.75
10	TMVW-t	MT20	4.0	8.0	1.75	Edge
11	BMV1+p	MT20	2.0	4.0	2.25	1.00
12	BMWW-t	MT20	4.0	5.0	1.50	1.50
13	BSWWW-l	MT20	5.0	8.0	3.00	4.00
14	BSWWW-l	MT20	5.0	8.0	3.00	4.00
15	BMWW-t	MT20	4.0	6.0	1.75	2.00
16	BMV1+p	MT20	2.0	4.0	2.25	1.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.



March 10, 2017

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
16	1925	0	1977	236	-924	5-8	5-8
11	1820	0	1852	0	-856	HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-8	

PROVIDE ANCHORAGE AT BEARING JOINT 16 FOR 924 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 11 FOR 856 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 236 LBS FACTORED HORIZONTAL REACTION AT JOINT 16

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
16	1349	957 / 0	0 / 0	0 / 0	132 / -912	392 / 0	0 / 0
11	1277	893 / 0	0 / 0	0 / 0	79 / -858	384 / 0	0 / 0
HORIZONTAL REACTIONS							
16	---	0 / 0	0 / 0	0 / 0	169 / -139	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 16

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.28 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 5.68 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 3-14, 5-14, 6-14, 6-13, 7-13, 9-13. DBS = 20-0-0 . CBF = 79 LBS.
1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 2-15. DBS = 10-0-0 . CBF = 79 LBS.
1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 10-12. DBS = 12-0-0 . CBF = 91 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 23	-77.3	-77.3	0.10 (1)	10.00	15- 3	-141 / 227	0.05 (3)
2- 3	-2859 / 1338	-77.3	-77.3	0.90 (1)	3.28	3-14	-674 / 558	0.37 (3)
3- 4	-2306 / 1166	-77.3	-77.3	0.84 (7)	3.72	14- 5	-285 / 653	0.14 (1)
4- 5	-2306 / 1166	-77.3	-77.3	0.84 (7)	3.72	14- 6	-208 / 277	0.13 (4)
5- 6	-2043 / 1143	-77.3	-77.3	0.31 (7)	4.63	6-13	-273 / 298	0.17 (3)
6- 7	-2001 / 1127	-77.3	-77.3	0.31 (8)	4.65	13- 7	-288 / 658	0.14 (1)
7- 8	-2266 / 1152	-77.3	-77.3	0.80 (8)	3.82	13- 9	-569 / 498	0.30 (4)
8- 9	-2266 / 1152	-77.3	-77.3	0.80 (8)	3.82	12- 9	-212 / 254	0.08 (1)
9-10	-2683 / 1270	-77.3	-77.3	0.81 (8)	3.47	2-15	-1048 / 2590	0.57 (1)
16- 2	-1920 / 961	0.0	0.0	0.19 (1)	6.13	12-10	-1026 / 2451	0.54 (1)
11-10	-1796 / 893	0.0	0.0	0.18 (1)	6.28			
16-15	-219 / 193	-17.5	-17.5	0.23 (11)	6.25			
15-14	-1261 / 2622	-17.5	-17.5	0.52 (1)	5.68			
14-13	-811 / 2131	-17.5	-17.5	0.45 (1)	6.25			
13-12	-994 / 2426	-17.5	-17.5	0.50 (1)	6.21			
12-11	-11 / 23	-17.5	-17.5	0.22 (11)	6.25			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.28")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (1.28")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.33")

CSI: TC=0.90 (2-3:1) , BC=0.52 (14-15:1) , WB=0.57 (2-15:1) , SSI=0.26 (2-3:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

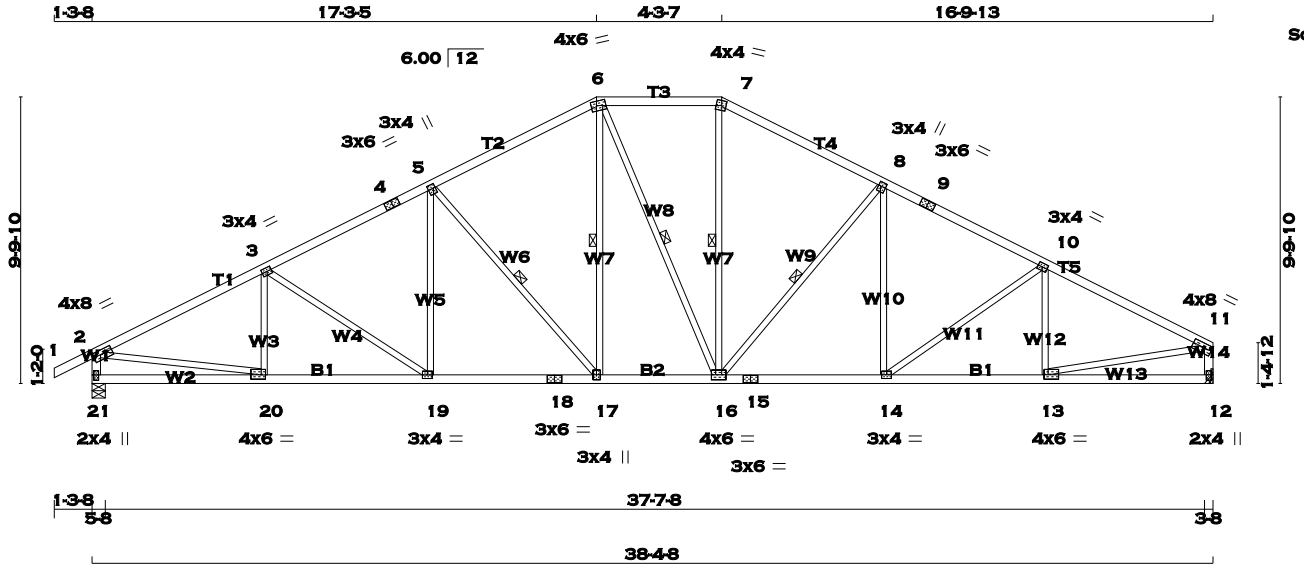
JSI GRIP= 0.90 (5) (INPUT = 0.90)
JSI METAL= 0.73 (12) (INPUT = 1.00)

CONTINUED ON PAGE 2

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



TOTAL WEIGHT = 169 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 6	2x4	DRY	No.2	SPF
6 - 7	2x4	DRY	No.2	SPF
7 - 9	2x4	DRY	No.2	SPF
9 - 11	2x4	DRY	No.2	SPF
21 - 2	2x4	DRY	No.2	SPF
12 - 11	2x4	DRY	No.2	SPF
21 - 18	2x4	DRY	No.2	SPF
18 - 15	2x4	DRY	No.2	SPF
15 - 12	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

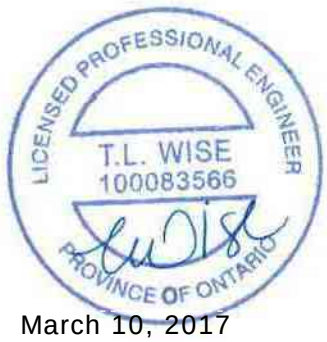
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-t	MT20	4.0	8.0	1.75	3.00
3	TMWW-t	MT20	3.0	4.0	1.50	1.75
4	TS-t	MT20	3.0	6.0		
5	TMWW+t	MT20	3.0	4.0	1.75	0.75
6	TTWW-m	MT20	4.0	6.0	1.75	2.25
7	TTW-m	MT20	4.0	4.0	2.00	1.75
8	TMWW+t	MT20	3.0	4.0	1.75	0.75
9	TS-t	MT20	3.0	6.0		
10	TMWW-t	MT20	3.0	4.0	1.50	1.75
11	TMVW-t	MT20	4.0	8.0	1.75	Edge
12	BMV1+p	MT20	2.0	4.0	2.25	1.00
13	BMWW-t	MT20	4.0	6.0	1.75	1.75
14	BMWW-t	MT20	3.0	4.0		
15	BS-t	MT20	3.0	6.0		
16	BMWWW-t	MT20	4.0	6.0		
17	BMWW+t	MT20	3.0	4.0		
18	BS-t	MT20	3.0	6.0		
19	BMWW-t	MT20	3.0	4.0		
20	BMWW-t	MT20	4.0	6.0	1.75	1.75
21	BMV1+p	MT20	2.0	4.0	2.25	1.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.



March 10, 2017

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
21	1925	0	1972	262	-896	5-8	5-8
12	1820	0	1857	0	-828	HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-8	

PROVIDE ANCHORAGE AT BEARING JOINT 21 FOR 896 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 12 FOR 828 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 262 LBS FACTORED HORIZONTAL REACTION AT JOINT 21

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
21	1349	957 / 0	0 / 0	0 / 0	117 / -892	392 / 0	0 / 0
12	1277	893 / 0	0 / 0	0 / 0	92 / -838	384 / 0	0 / 0
HORIZONTAL REACTIONS							
21	---	0 / 0	0 / 0	0 / 0	187 / -153	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 21

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.82 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 5.74 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 5-17, 6-17, 6-16, 7-16, 8-16. DBS = 20-0-0 . CBF = 84 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 23	-77.3	-77.3	0.10 (1)	10.00	20- 3	-260 / 231	0.06 (1)
2- 3	-2803 / 1264	-77.3	-77.3	0.48 (7)	3.82	3-19	-268 / 284	0.23 (3)
3- 4	-2586 / 1241	-77.3	-77.3	0.48 (7)	4.05	19- 5	-88 / 253	0.07 (7)
4- 5	-2586 / 1241	-77.3	-77.3	0.48 (7)	4.05	5-17	-711 / 541	0.37 (3)
5- 6	-2098 / 1080	-77.3	-77.3	0.45 (7)	4.40	17- 6	-344 / 618	0.22 (7)
6- 7	-1849 / 1038	-77.3	-77.3	0.32 (8)	4.77	6-16	-132 / 150	0.10 (8)
7- 8	-2082 / 1073	-77.3	-77.3	0.43 (8)	4.43	16- 7	-255 / 581	0.16 (7)
8- 9	-2492 / 1206	-77.3	-77.3	0.46 (8)	4.11	16- 8	-643 / 505	0.33 (4)
9-10	-2492 / 1206	-77.3	-77.3	0.46 (8)	4.11	14- 8	-66 / 197	0.06 (8)
10-11	-2616 / 1183	-77.3	-77.3	0.46 (8)	3.96	14-10	-171 / 233	0.14 (4)
21- 2	-1926 / 923	0.0	0.0	0.19 (1)	6.12	13-10	-338 / 264	0.09 (1)
12-11	-1813 / 855	0.0	0.0	0.18 (1)	6.26	2-20	-1006 / 2540	0.58 (7)
						13-11	-973 / 2384	0.53 (8)
21-20	-245 / 219	-17.5	-17.5	0.13 (11)	6.25			
20-19	-1241 / 2567	-17.5	-17.5	0.45 (1)	5.74			
19-18	-1005 / 2344	-17.5	-17.5	0.42 (1)	6.23			
18-17	-1005 / 2344	-17.5	-17.5	0.42 (1)	6.23			
17-16	-654 / 1884	-17.5	-17.5	0.35 (1)	6.25			
16-15	-745 / 2230	-17.5	-17.5	0.41 (1)	6.25			
15-14	-745 / 2230	-17.5	-17.5	0.41 (1)	6.25			
14-13	-935 / 2335	-17.5	-17.5	0.42 (1)	6.25			
13-12	-11 / 23	-17.5	-17.5	0.12 (11)	6.25			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.28")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (1.28")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.28")

CSI: TC=0.48 (2-3:7) , BC=0.45 (19-20:1) , WB=0.58 (2-20:7) , SSI=0.19 (2-3:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

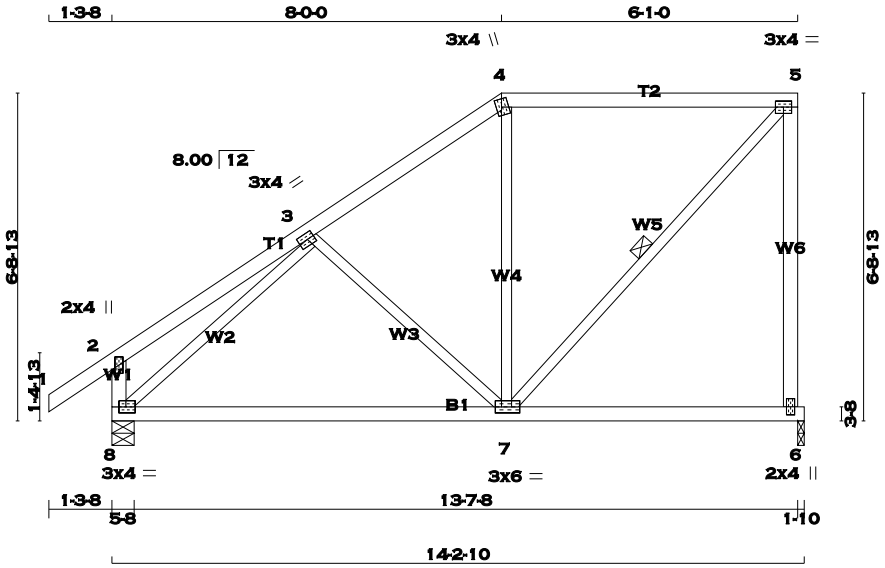
JSI GRIP= 0.90 (7) (INPUT = 0.90)
JSI METAL= 0.63 (18) (INPUT = 1.00)

CONTINUED ON PAGE 2

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



TOTAL WEIGHT = 64 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 5	2x4	DRY	No.2	SPF
6 - 5	2x4	DRY	No.2	SPF
8 - 2	2x4	DRY	No.2	SPF
8 - 6	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMV+p	MT20	2.0	4.0		
3	TMWW-t	MT20	3.0	4.0	1.50	1.75
4	TTW+m	MT20	3.0	4.0	2.00	1.25
5	TMVW-t	MT20	3.0	4.0		
6	BMV1+p	MT20	2.0	4.0		
7	BMWWW-t	MT20	3.0	6.0	1.50	1.50
8	BMVW1-t	MT20	3.0	4.0	1.50	1.75

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
6	668	0	726	0	-397	1-10	1-10
8	774	0	830	384	-372	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 6 FOR 397 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 8 FOR 372 LBS FACTORED UPLIFT

PROVIDE FOR 384 LBS FACTORED HORIZONTAL REACTION AT JOINT 8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
6	469	328 / 0	0 / 0	0 / 0	145 / -374	141 / 0	0 / 0
8	541	392 / 0	0 / 0	0 / 0	139 / -361	149 / 0	0 / 0
HORIZONTAL REACTIONS							
8	---	0 / 0	0 / 0	0 / 0	274 / -181	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 6, 8

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 5-7. DBS = 20-0-0 . CBF = 34 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 29	-77.3	-77.3	0.10 (1)	10.00	3- 7	-267 / 357	0.14 (3)
2- 3	-26 / 167	-77.3	-77.3	0.24 (7)	6.25	7- 4	-111 / 128	0.08 (3)
3- 4	-510 / 326	-77.3	-77.3	0.26 (7)	6.25	7- 5	-315 / 607	0.15 (7)
4- 5	-415 / 340	-77.3	-77.3	0.39 (7)	6.25	8- 3	-756 / 236	0.36 (4)
6- 5	-692 / 420	0.0	0.0	0.61 (1)	7.81			
8- 2	-260 / 250	0.0	0.0	0.04 (7)	7.81			
8- 7	-378 / 610	-17.5	-17.5	0.30 (11)	6.25			
7- 6	-49 / 126	-17.5	-17.5	0.27 (11)	6.25			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.47")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.47")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.61 (5-6:1) , BC=0.30 (7-8:11) , WB=0.36 (3-8:4) , SSI=0.18 (4-5:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (7) (INPUT = 0.90)
JSI METAL= 0.26 (3) (INPUT = 1.00)

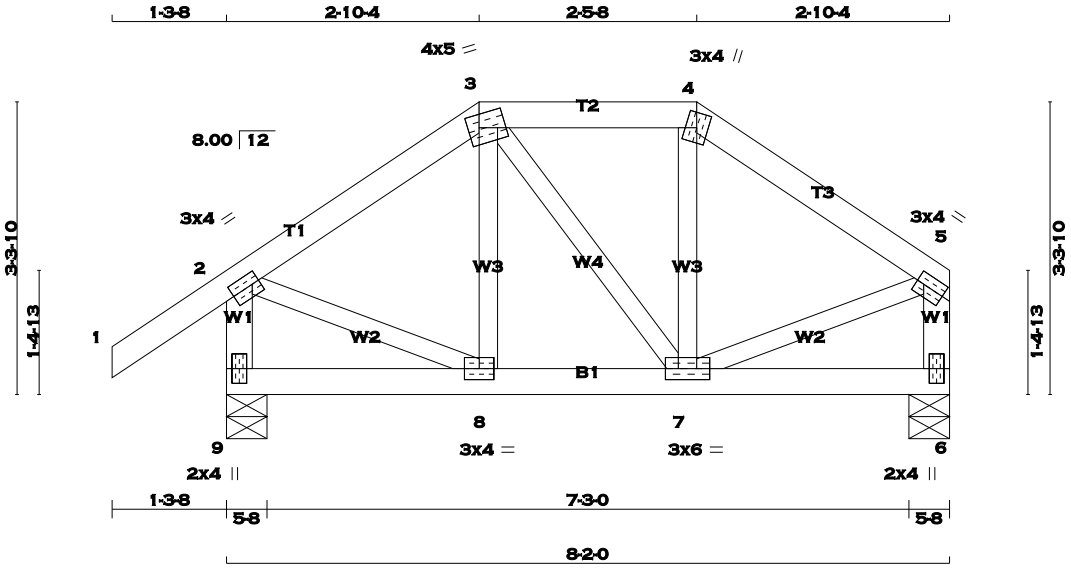
LICENSED PROFESSIONAL ENGINEER
T.L. WISE
100083566
PROVINCE OF ONTARIO

March 10, 2017

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

RECEIVED
TOWN OF MILTON
MAR 29, 2017
17-4978
BUILDING DIVISION



TOTAL WEIGHT = 36 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 3	2x4	DRY	No.2	SPF
3 - 4	2x4	DRY	No.2	SPF
4 - 5	2x4	DRY	No.2	SPF
9 - 2	2x4	DRY	No.2	SPF
6 - 5	2x4	DRY	No.2	SPF
9 - 6	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-t	MT20	3.0	4.0	1.50	1.00
3	TTWW-m	MT20	4.0	5.0	1.75	1.50
4	TTW+m	MT20	3.0	4.0		
5	TMVW-t	MT20	3.0	4.0	1.50	1.00
6	BMV1+p	MT20	2.0	4.0		
7	BMWWW-t	MT20	3.0	6.0		
8	BMWW-t	MT20	3.0	4.0		
9	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
9	493	0	521	0	-237	5-8	5-8
6	387	0	403	161	-182	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 9 FOR 237 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 6 FOR 182 LBS FACTORED UPLIFT

PROVIDE FOR 161 LBS FACTORED HORIZONTAL REACTION AT JOINT 6

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
9	344	254 / 0	0 / 0	0 / 0	68 / -227	90 / 0	0 / 0
6	272	190 / 0	0 / 0	0 / 0	39 / -182	82 / 0	0 / 0
HORIZONTAL REACTIONS							
6	---	0 / 0	0 / 0	0 / 0	115 / -107	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 9, 6

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 29	-77.3	-77.3	0.10 (1)	10.00	8- 3	-46 / 55	0.01 (8)
2- 3	-313 / 171	-77.3	-77.3	0.14 (7)	6.25	3- 7	-55 / 58	0.01 (5)
3- 4	-260 / 196	-77.3	-77.3	0.08 (8)	6.25	7- 4	-50 / 77	0.01 (8)
4- 5	-314 / 171	-77.3	-77.3	0.13 (8)	6.25	2- 8	-60 / 277	0.06 (1)
9- 2	-498 / 252	0.0	0.0	0.05 (1)	7.81	7- 5	-85 / 278	0.06 (1)
6- 5	-380 / 196	0.0	0.0	0.04 (1)	7.81			
9- 8	-10 / 21	-17.5	-17.5	0.03 (11)	10.00			
8- 7	-48 / 258	-17.5	-17.5	0.05 (1)	6.25			
7- 6	-128 / 151	-17.5	-17.5	0.03 (6)	6.25			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.27")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.27")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.14 (2-3:7) , BC=0.05 (7-8:1) , WB=0.06 (2-8:1) , SSI=0.08 (4-5:8)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

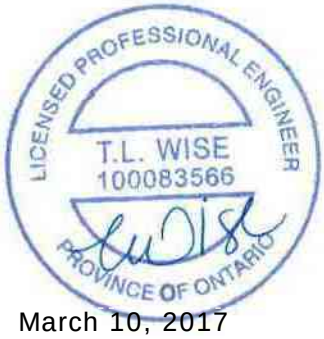
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (2) (INPUT = 0.90)
JSI METAL= 0.16 (2) (INPUT = 1.00)



March 10, 2017

KOTT

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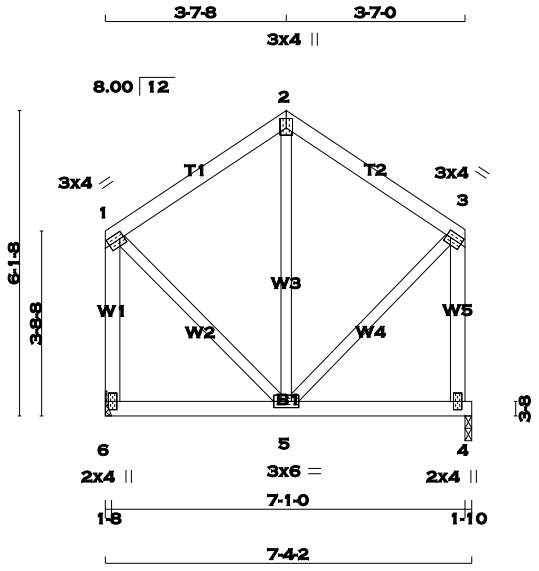


SCALE = 1:46.2

TOTAL WEIGHT = 2 X 38 = 76 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 2	2x4	DRY	No.2	SPF
2 - 3	2x4	DRY	No.2	SPF
6 - 1	2x4	DRY	No.2	SPF
4 - 3	2x4	DRY	No.2	SPF
6 - 4	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
1	TMVW-t	MT20	3.0	4.0	1.50	1.00
2	TTW+p	MT20	3.0	4.0	2.25	1.50
3	TMVW-t	MT20	3.0	4.0	1.50	1.00
4	BMV1+p	MT20	2.0	4.0		
5	BMWWW-t	MT20	3.0	6.0		
6	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
6	342	0	380	290	-149	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
4	342	0	380	0	-150	1-10	1-10

PROVIDE ANCHORAGE AT BEARING JOINT 6 FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 4 FOR 150 LBS FACTORED UPLIFT

PROVIDE FOR 290 LBS FACTORED HORIZONTAL REACTION AT JOINT 6

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
6	240	168 / 0	0 / 0	0 / 0	94 / -153	72 / 0	0 / 0
4	240	168 / 0	0 / 0	0 / 0	95 / -154	72 / 0	0 / 0
HORIZONTAL REACTIONS							
6	---	0 / 0	0 / 0	0 / 0	207 / -207	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 4

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	-161 / 157	-77.3	-77.3	0.20 (7)	6.25	5- 2	-131 / 25	0.08 (1)
2- 3	-161 / 158	-77.3	-77.3	0.19 (8)	6.25	1- 5	-108 / 185	0.04 (5)
6- 1	-355 / 166	0.0	0.0	0.09 (8)	7.81	5- 3	-109 / 187	0.04 (6)
4- 3	-355 / 167	0.0	0.0	0.09 (7)	7.81			
6- 5	-224 / 259	-17.5	-17.5	0.06 (11)	6.25			
5- 4	-31 / 67	-17.5	-17.5	0.06 (11)	6.25			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	23.3	PSF
		DL =	3.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.0	PSF
TOTAL	LOAD	=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.24")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.24")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.20 (1-2:7) , BC=0.06 (4-5:11) , WB=0.08 (2-5:1) , SSI=0.10 (1-2:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

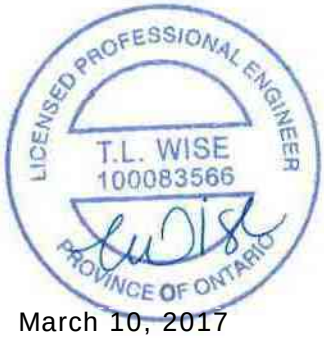
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.51 (3) (INPUT = 0.90)
JSI METAL= 0.09 (1) (INPUT = 1.00)



March 10, 2017

KOTT

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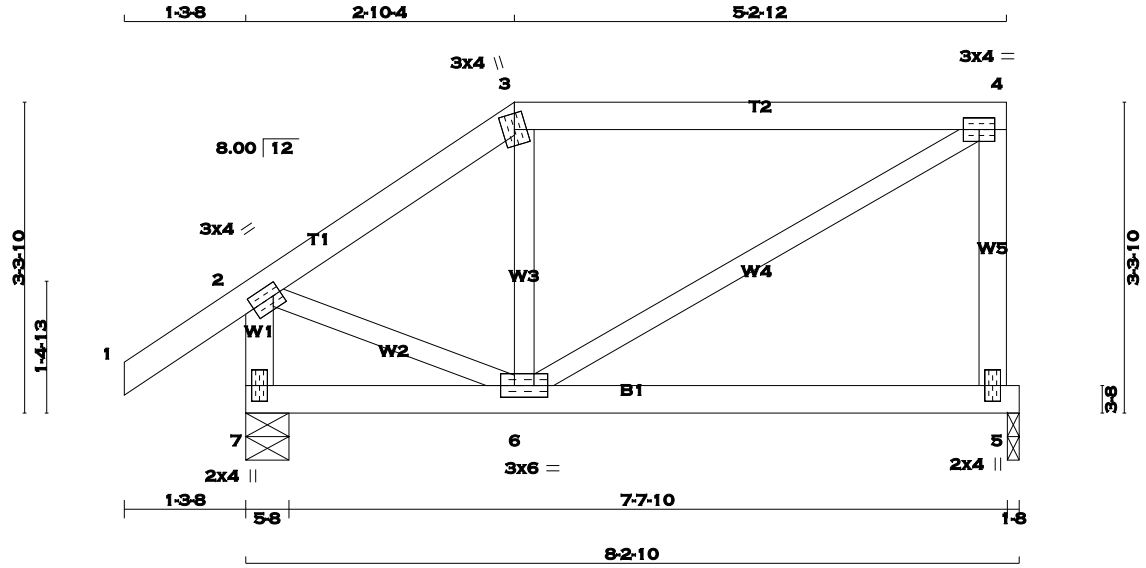


TOTAL WEIGHT = 34 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 3	2x4	DRY	No.2	SPF
3 - 4	2x4	DRY	No.2	SPF
5 - 4	2x4	DRY	No.2	SPF
7 - 2	2x4	DRY	No.2	SPF
7 - 5	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-t	MT20	3.0	4.0	1.50	1.00
3	TTW+m	MT20	3.0	4.0		
4	TMVW-t	MT20	3.0	4.0		
5	BMV1+p	MT20	2.0	4.0		
6	BMWWW-t	MT20	3.0	6.0		
7	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
5	383	0	409	0	-223	1-8	1-8
7	490	0	520	191	-246	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 5 FOR 223 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 7 FOR 246 LBS FACTORED UPLIFT

PROVIDE FOR 191 LBS FACTORED HORIZONTAL REACTION AT JOINT 7

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
5	269	188 / 0	0 / 0	0 / 0	65 / -211	81 / 0	0 / 0
7	341	252 / 0	0 / 0	0 / 0	76 / -233	89 / 0	0 / 0
HORIZONTAL REACTIONS							
7	---	0 / 0	0 / 0	0 / 0	136 / -98	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 5, 7

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 29	-77.3	-77.3	0.10 (1)	10.00	6- 3	-159 / 158	0.03 (7)
2- 3	-323 / 172	-77.3	-77.3	0.14 (7)	6.25	6- 4	-168 / 315	0.09 (7)
3- 4	-272 / 203	-77.3	-77.3	0.37 (1)	6.25	2- 6	-35 / 279	0.06 (1)
5- 4	-372 / 248	0.0	0.0	0.09 (7)	7.81			
7- 2	-506 / 253	0.0	0.0	0.05 (1)	7.81			
7- 6	-169 / 127	-17.5	-17.5	0.10 (11)	6.25			
6- 5	-23 / 59	-17.5	-17.5	0.10 (11)	6.25			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.27")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.27")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.37 (3-4:1) , BC=0.10 (6-7:11) , WB=0.09 (4-6:7) , SSI=0.16 (3-4:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

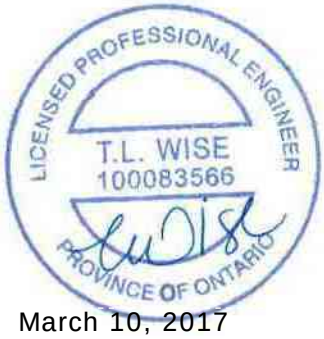
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (2) (INPUT = 0.90)
JSI METAL= 0.17 (2) (INPUT = 1.00)



March 10, 2017

KOTT

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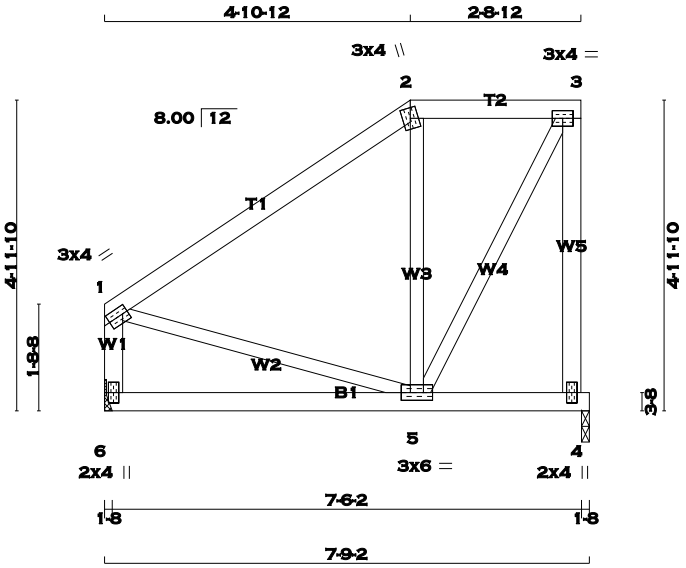


SCALE = 1:36.9

TOTAL WEIGHT = 37 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 2	2x4	DRY	No.2	SPF
2 - 3	2x4	DRY	No.2	SPF
4 - 3	2x4	DRY	No.2	SPF
6 - 1	2x4	DRY	No.2	SPF
6 - 4	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
1	TMVW-t	MT20	3.0	4.0	1.50	1.00
2	TTW+m	MT20	3.0	4.0		
3	TMVW-t	MT20	3.0	4.0		
4	BMV1+p	MT20	2.0	4.0		
5	BMWWW-t	MT20	3.0	6.0		
6	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
4	362	0	402	0	-228	1-8	1-8
6	362	0	384	260	-153	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

PROVIDE ANCHORAGE AT BEARING JOINT 4 FOR 228 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 6 FOR 153 LBS FACTORED UPLIFT

PROVIDE FOR 260 LBS FACTORED HORIZONTAL REACTION AT JOINT 6

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
4	254	178 / 0	0 / 0	0 / 0	100 / -212	76 / 0	0 / 0
6	254	178 / 0	0 / 0	0 / 0	55 / -158	76 / 0	0 / 0
HORIZONTAL REACTIONS							
6	---	0 / 0	0 / 0	0 / 0	185 / -134	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 4

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	-199 / 80	-77.3	-77.3	0.34 (7)	6.25	5- 2	-228 / 195	0.08 (3)
2- 3	-160 / 174	-77.3	-77.3	0.09 (7)	6.25	5- 3	-170 / 323	0.07 (7)
4- 3	-392 / 233	0.0	0.0	0.16 (7)	7.81	1- 5	-46 / 173	0.03 (4)
6- 1	-347 / 178	0.0	0.0	0.04 (7)	7.81			
6- 5	-232 / 175	-17.5	-17.5	0.09 (11)	6.25			
5- 4	-36 / 92	-17.5	-17.5	0.09 (11)	6.25			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.25")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.25")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.34 (1-2:7) , BC=0.09 (4-5:11) , WB=0.08 (2-5:3) , SSI=0.13 (1-2:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

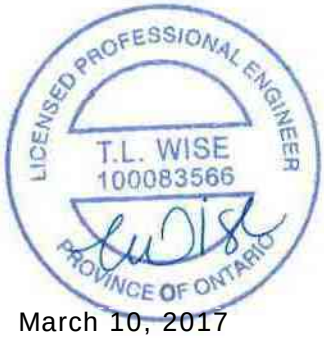
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.60 (5) (INPUT = 0.90)
JSI METAL= 0.11 (1) (INPUT = 1.00)



March 10, 2017

KOTT

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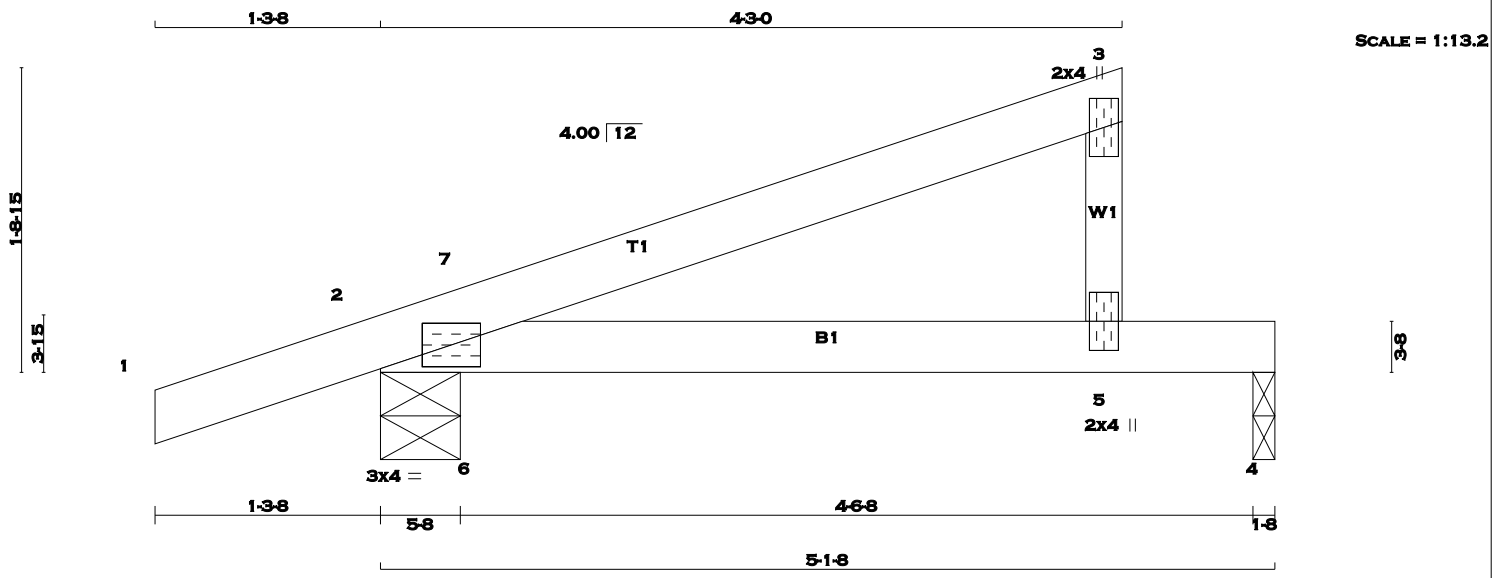

TOTAL WEIGHT = 6 X 14 = 81 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 3	2x4	DRY	No.2	SPF
5 - 3	2x3	DRY	No.2	SPF
2 - 4	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMB1-I	MT20	3.0	4.0		
3	TMV+p	MT20	2.0	4.0		
5	BMV+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
2	341	0	341	117	-182	5-8	5-8
4	181	0	181	0	-84	1-8	1-8

PROVIDE ANCHORAGE AT BEARING JOINT 2 FOR 182 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 4 FOR 150 LBS FACTORED UPLIFT

PROVIDE FOR 117 LBS FACTORED HORIZONTAL REACTION AT JOINT 2

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
2	237	178 / 0	0 / 0	0 / 0	0 / -168	59 / 0	0 / 0
4	129	82 / 0	0 / 0	0 / 0	0 / -90	46 / 0	0 / 0
HORIZONTAL REACTIONS							
2	---	0 / 0	0 / 0	0 / 0	84 / 0	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 2, 4

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 15	-77.3	-77.3	0.10 (1)	10.00	6- 7	-71 / 165	0.00 (1)
2- 7	-92 / 0	-77.3	-77.3	0.08 (11)	6.25			
7- 3	-48 / 4	-77.3	-77.3	0.21 (1)	6.25			
5- 3	-154 / 117	0.0	0.0	0.05 (5)	7.81			
2- 6	-16 / 33	-17.5	-17.5	0.11 (1)	6.25			
6- 5	-16 / 33	-17.5	-17.5	0.21 (1)	6.25			
5- 4	0 / 0	-17.5	-17.5	0.21 (1)	10.00			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 694 (0.09")

CSI: TC=0.21 (3-7:1) , BC=0.21 (5-6:1) , WB=0.00 (6-7:1) , SSI=0.14 (4-5:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (2) (INPUT = 0.90)
JSI METAL= 0.05 (2) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
T.L. WISE
100083566
PROVINCE OF ONTARIO
March 10, 2017

KOTT

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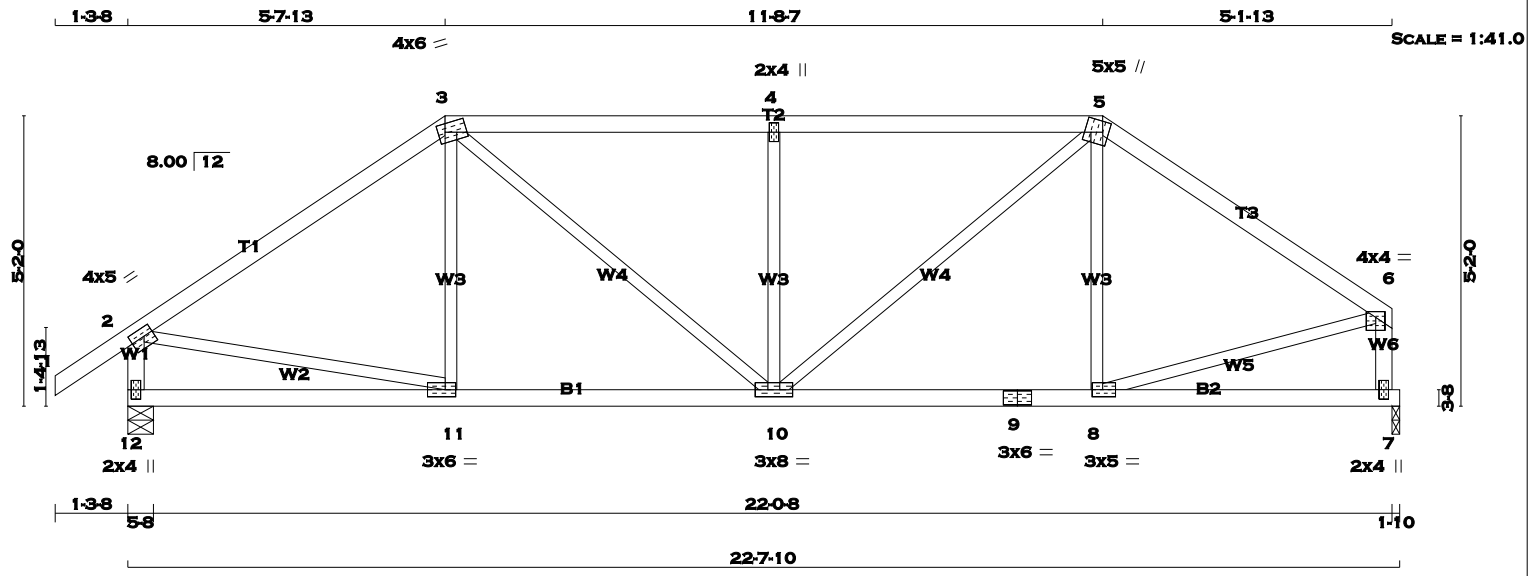
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
TW0317-048	**T70**	**1**	**1**	TR-GREENPARK-LECCO RIDGE-BLOCK 327 TRUSS DESC.	Page 144 of 159 TW0317-048

Kott Lumber Uxbridge, Stouffville, ON, TW

Version 8.100 S Feb 9 2017 MiTek Industries, Inc. Fri Mar 10 14:21:06 2017 Page 1

ID:4BORDhR74Ei?Dog_xdfUkJyKZi_-h2Nsltv4IVt0lDC4lDhbryxuiOXW_ndrUXXN2MzcJJR



TOTAL WEIGHT = 90 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 3	2x4	DRY	No.2	SPF
3 - 5	2x4	DRY	No.2	SPF
5 - 6	2x4	DRY	No.2	SPF
12 - 2	2x4	DRY	No.2	SPF
7 - 6	2x4	DRY	No.2	SPF
12 - 9	2x4	DRY	No.2	SPF
9 - 7	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-t	MT20	4.0	5.0	1.75	2.00
3	TTWW-m	MT20	4.0	6.0	1.75	1.50
4	TMW+w	MT20	2.0	4.0		
5	TTWW+m	MT20	5.0	5.0	2.25	1.25
6	TMVW-p	MT20	4.0	4.0	1.25	2.00
7	BMV1+p	MT20	2.0	4.0		
8	BMWW-t	MT20	3.0	5.0	1.50	2.25
9	BS-t	MT20	3.0	6.0		
10	BMWWW-t	MT20	3.0	8.0		
11	BMWW-t	MT20	3.0	6.0	1.50	2.25
12	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
12	1173	0	1247	241	-596	5-8	5-8
7	1067	0	1101	0	-533	1-10	1-10

PROVIDE ANCHORAGE AT BEARING JOINT 12 FOR 596 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 7 FOR 533 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 241 LBS FACTORED HORIZONTAL REACTION AT JOINT 12

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
12	821	588 / 0	0 / 0	0 / 0	183 / -576	233 / 0	0 / 0
7	749	524 / 0	0 / 0	0 / 0	86 / -525	225 / 0	0 / 0
HORIZONTAL REACTIONS							
12	---	0 / 0	0 / 0	0 / 0	172 / -158	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 12, 7

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.99 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 29	-77.3	-77.3	0.10 (1)	10.00	11- 3	-66 / 144	0.03 (8)
2- 3	-1198 / 591	-77.3	-77.3	0.52 (7)	5.20	3-10	-325 / 481	0.36 (8)
3- 4	-1374 / 794	-77.3	-77.3	0.50 (1)	4.99	10- 4	-584 / 451	0.22 (3)
4- 5	-1374 / 794	-77.3	-77.3	0.50 (1)	4.99	10- 5	-364 / 590	0.40 (7)
5- 6	-1113 / 559	-77.3	-77.3	0.44 (8)	5.55	8- 5	-139 / 170	0.05 (3)
12- 2	-1206 / 625	0.0	0.0	0.12 (1)	7.33	2-11	-341 / 980	0.22 (1)
7- 6	-1066 / 558	0.0	0.0	0.11 (1)	7.67	8- 6	-357 / 959	0.21 (1)
12-11	-220 / 212	-17.5	-17.5	0.14 (11)	6.25			
11-10	-417 / 1016	-17.5	-17.5	0.23 (1)	6.25			
10- 9	-314 / 923	-17.5	-17.5	0.21 (1)	6.25			
9- 8	-314 / 923	-17.5	-17.5	0.21 (1)	6.25			
8- 7	-13 / 28	-17.5	-17.5	0.13 (11)	6.25			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.75")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.75")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.52 (2-3:7) , BC=0.23 (10-11:1) , WB=0.40 (5-10:7) , SSI=0.22 (3-4:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (6) (INPUT = 0.90)
JSI METAL= 0.41 (2) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
T.L. WISE
100083566
PROVINCE OF ONTARIO

March 10, 2017

KOTT

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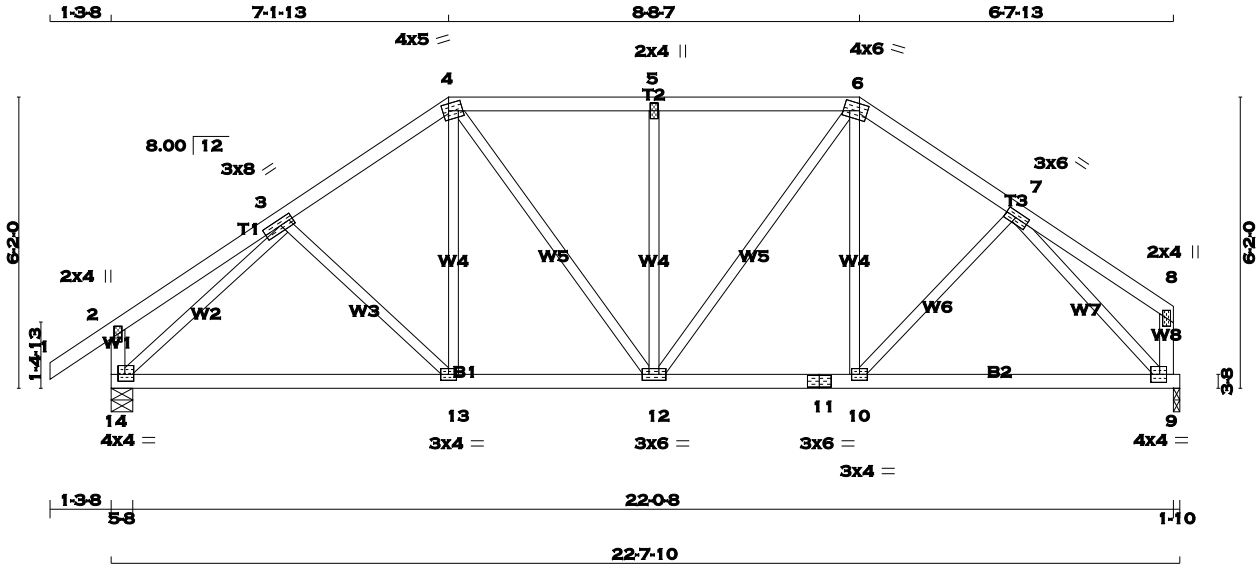


TOTAL WEIGHT = 99 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 6	2x4	DRY	No.2	SPF
6 - 8	2x4	DRY	No.2	SPF
14 - 2	2x4	DRY	No.2	SPF
9 - 8	2x4	DRY	No.2	SPF
14 - 11	2x4	DRY	No.2	SPF
11 - 9	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMV+p	MT20	2.0	4.0		
3	TMWW-t	MT20	3.0	8.0	1.50	3.50
4	TTWW-m	MT20	4.0	5.0	1.75	1.50
5	TMV+w	MT20	2.0	4.0		
6	TTWW-m	MT20	4.0	6.0	1.75	2.00
7	TMWW-t	MT20	3.0	6.0	1.50	2.75
8	TMV+p	MT20	2.0	4.0		
9	BMVW1-t	MT20	4.0	4.0	2.00	1.75
10	BMWW-t	MT20	3.0	4.0		
11	BS-t	MT20	3.0	6.0		
12	BMWWW-t	MT20	3.0	6.0		
13	BMWW-t	MT20	3.0	4.0		
14	BMVW1-t	MT20	4.0	4.0	1.75	1.75

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
14	1173	0	1249	281	-580	5-8	5-8
9	1067	0	1110	0	-515	1-10	1-10

PROVIDE ANCHORAGE AT BEARING JOINT 14 FOR 580 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 9 FOR 515 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 281 LBS FACTORED HORIZONTAL REACTION AT JOINT 14

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
14	821	588 / 0	0 / 0	0 / 0	189 / -564	233 / 0	0 / 0
9	749	524 / 0	0 / 0	0 / 0	106 / -513	225 / 0	0 / 0
HORIZONTAL REACTIONS							
14	---	0 / 0	0 / 0	0 / 0	200 / -186	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 14, 9

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.79 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
1- 2	0 / 29	-77.3	-77.3	0.10 (1)	10.00
2- 3	-25 / 159	-77.3	-77.3	0.19 (7)	6.25
3- 4	-1175 / 649	-77.3	-77.3	0.26 (7)	5.79
4- 5	-1113 / 674	-77.3	-77.3	0.26 (7)	5.85
5- 6	-1113 / 674	-77.3	-77.3	0.26 (7)	5.85
6- 7	-1105 / 626	-77.3	-77.3	0.23 (8)	5.96
7- 8	-23 / 104	-77.3	-77.3	0.16 (8)	6.25
14- 2	-248 / 237	0.0	0.0	0.04 (7)	7.81
9- 8	-128 / 137	0.0	0.0	0.03 (8)	7.81
14-13	-544 / 1044	-17.5	-17.5	0.26 (11)	6.25
13-12	-362 / 978	-17.5	-17.5	0.27 (11)	6.25
12-11	-275 / 909	-17.5	-17.5	0.24 (11)	6.25
11-10	-275 / 909	-17.5	-17.5	0.24 (11)	6.25
10- 9	-350 / 873	-17.5	-17.5	0.23 (1)	6.25

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
3-13	-94 / 251	0.05 (7)
13- 4	-75 / 196	0.04 (7)
4-12	-235 / 266	0.23 (8)
12- 5	-430 / 332	0.25 (3)
12- 6	-264 / 348	0.26 (7)
10- 6	-46 / 134	0.03 (11)
10- 7	-50 / 188	0.04 (8)
14- 3	-1362 / 545	0.51 (1)
7- 9	-1292 / 560	0.48 (1)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.75")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.75")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.26 (4-5:7) , BC=0.27 (12-13:11) , WB=0.51 (3-14:1) , SSI=0.16 (4-5:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

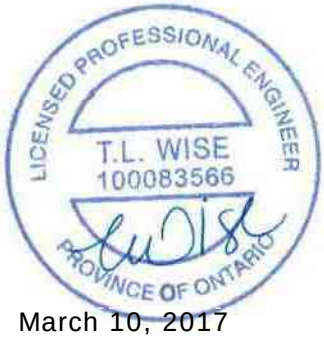
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (4) (INPUT = 0.90)
JSI METAL= 0.36 (14) (INPUT = 1.00)



March 10, 2017

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

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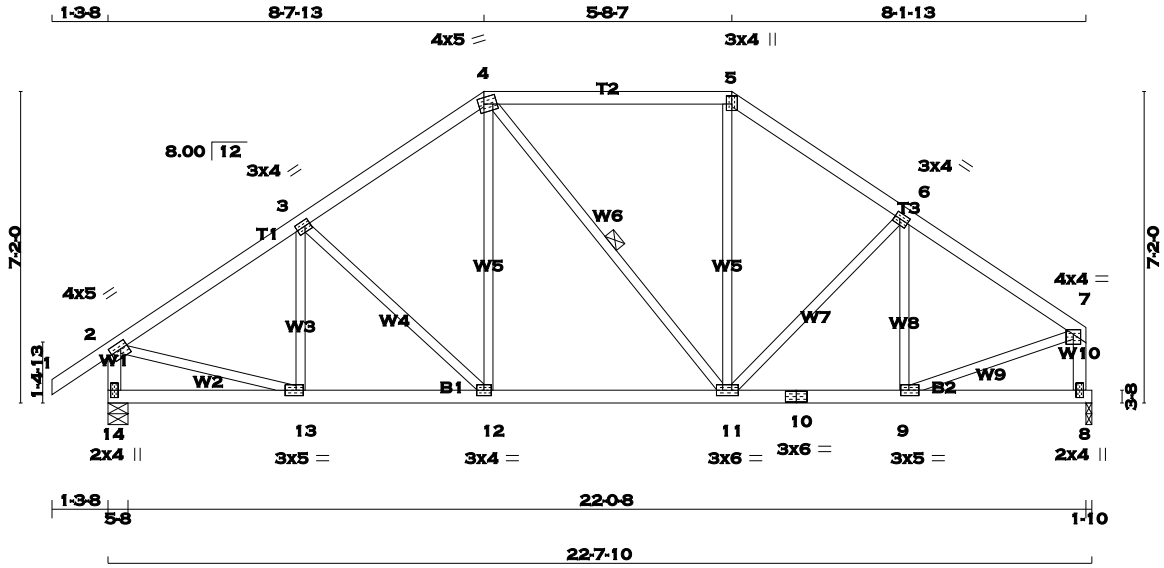

TOTAL WEIGHT = 100 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 5	2x4	DRY	No.2	SPF
5 - 7	2x4	DRY	No.2	SPF
14 - 2	2x4	DRY	No.2	SPF
8 - 7	2x4	DRY	No.2	SPF
14 - 10	2x4	DRY	No.2	SPF
10 - 8	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-t	MT20	4.0	5.0	1.75	2.00
3	TMWW-t	MT20	3.0	4.0	1.50	1.50
4	TTWW-m	MT20	4.0	5.0	1.75	1.50
5	TTW+p	MT20	3.0	4.0	2.25	1.50
6	TMWW-t	MT20	3.0	4.0	1.50	1.50
7	TMVW-p	MT20	4.0	4.0	1.25	2.00
8	BMV1+p	MT20	2.0	4.0		
9	BMWW-t	MT20	3.0	5.0	1.50	2.25
10	BS-t	MT20	3.0	6.0		
11	BMWWW-t	MT20	3.0	6.0		
12	BMWW-t	MT20	3.0	4.0		
13	BMWW-t	MT20	3.0	5.0	1.50	2.00
14	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
14	1173	0	1250	320	-560	5-8	5-8
8	1067	0	1119	0	-494	1-10	1-10

PROVIDE ANCHORAGE AT BEARING JOINT 14 FOR 560 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 8 FOR 494 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 320 LBS FACTORED HORIZONTAL REACTION AT JOINT 14

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
14	821	588 / 0	0 / 0	0 / 0	191 / -550	233 / 0	0 / 0
8	749	524 / 0	0 / 0	0 / 0	130 / -498	225 / 0	0 / 0
HORIZONTAL REACTIONS							
14	---	0 / 0	0 / 0	0 / 0	229 / -214	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 14, 8

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 4-11. DBS = 20-0-0 . CBF = 14 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 29	-77.3	-77.3	0.10 (1)	10.00	13- 3	-173 / 146	0.05 (1)
2- 3	-1245 / 589	-77.3	-77.3	0.33 (7)	5.58	3-12	-277 / 304	0.16 (3)
3- 4	-1092 / 614	-77.3	-77.3	0.34 (7)	5.86	12- 4	-145 / 278	0.13 (7)
4- 5	-862 / 569	-77.3	-77.3	0.38 (8)	6.24	4-11	-132 / 138	0.07 (5)
5- 6	-1052 / 601	-77.3	-77.3	0.31 (8)	5.99	11- 5	-87 / 228	0.08 (7)
6- 7	-1118 / 537	-77.3	-77.3	0.30 (8)	5.83	11- 6	-193 / 252	0.11 (4)
14- 2	-1215 / 581	0.0	0.0	0.12 (1)	7.30	9- 6	-247 / 174	0.07 (1)
8- 7	-1086 / 515	0.0	0.0	0.11 (1)	7.61	2-13	-353 / 1059	0.23 (1)
						9- 7	-364 / 995	0.21 (1)
14-13	-299 / 290	-17.5	-17.5	0.07 (11)	6.25			
13-12	-522 / 1106	-17.5	-17.5	0.21 (1)	6.25			
12-11	-302 / 908	-17.5	-17.5	0.18 (1)	6.25			
11-10	-318 / 942	-17.5	-17.5	0.20 (1)	6.25			
10- 9	-318 / 942	-17.5	-17.5	0.20 (1)	6.25			
9- 8	-13 / 28	-17.5	-17.5	0.06 (11)	6.25			

WIND LOAD APPLIED IS BASED ON REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-SX ABOVE GROUND HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, GCp, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM} INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE OCCUPIED {OPEN} FRO... LEAST {0-0} FT-IN-SX AWAY

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.75")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.75")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.38 (4-5:8) , BC=0.21 (12-13:1) , WB=0.23 (2-13:1) , SSI=0.17 (4-5:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (5) (INPUT = 0.90)
JSI METAL= 0.42 (2) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
T.L. WISE
100083566
PROVINCE OF ONTARIO

March 10, 2017

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

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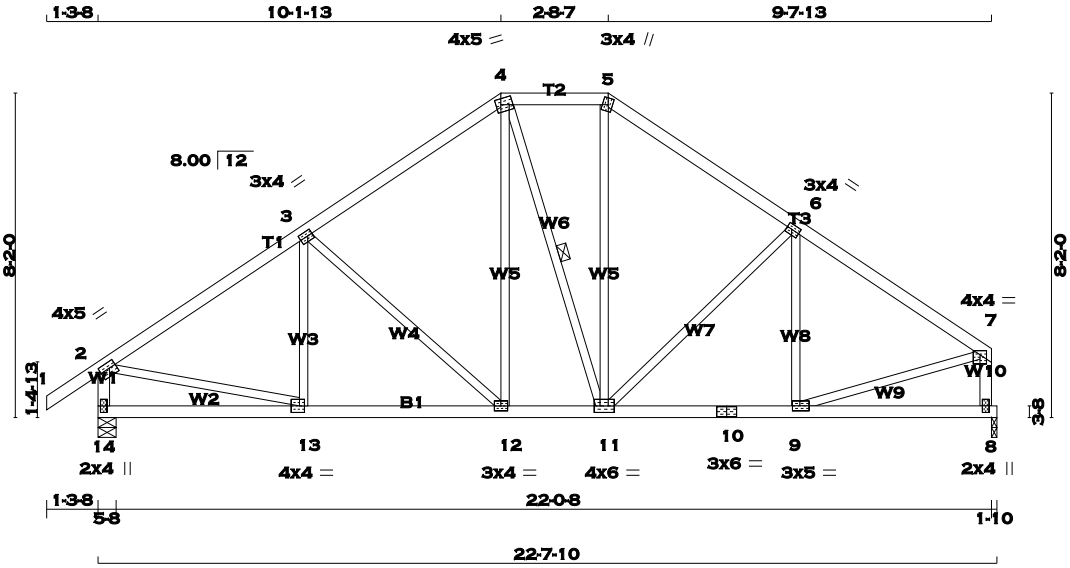

TOTAL WEIGHT = 105 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 5	2x4	DRY	No.2	SPF
5 - 7	2x4	DRY	No.2	SPF
14 - 2	2x4	DRY	No.2	SPF
8 - 7	2x4	DRY	No.2	SPF
14 - 10	2x4	DRY	No.2	SPF
10 - 8	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-t	MT20	4.0	5.0	1.75	2.00
3	TMWW-t	MT20	3.0	4.0	1.50	1.50
4	TTWW-m	MT20	4.0	5.0	1.75	1.50
5	TTW+m	MT20	3.0	4.0	2.00	1.25
6	TMWW-t	MT20	3.0	4.0	1.50	1.50
7	TMVW-p	MT20	4.0	4.0	1.25	2.00
8	BMV1+p	MT20	2.0	4.0		
9	BMWW-t	MT20	3.0	5.0	1.50	2.25
10	BS-t	MT20	3.0	6.0		
11	BMWWW-t	MT20	4.0	6.0		
12	BMWW-t	MT20	3.0	4.0		
13	BMWW-t	MT20	4.0	4.0	2.00	1.50
14	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
14	1173	0	1249	359	-535	5-8	5-8
8	1067	0	1127	0	-469	1-10	1-10

PROVIDE ANCHORAGE AT BEARING JOINT 14 FOR 535 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 8 FOR 469 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 359 LBS FACTORED HORIZONTAL REACTION AT JOINT 14

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
14	821	588 / 0	0 / 0	0 / 0	189 / -532	233 / 0	0 / 0
8	749	524 / 0	0 / 0	0 / 0	149 / -480	225 / 0	0 / 0
HORIZONTAL REACTIONS							
14	---	0 / 0	0 / 0	0 / 0	257 / -242	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 14, 8

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.46 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 4-11. DBS = 20-0-0 . CBF = 16 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 29	-77.3	-77.3	0.10 (1)	10.00	13- 3	-115 / 144	0.04 (4)
2- 3	-1252 / 557	-77.3	-77.3	0.43 (7)	5.46	3-12	-404 / 385	0.34 (3)
3- 4	-996 / 555	-77.3	-77.3	0.42 (7)	5.96	12- 4	-209 / 328	0.27 (7)
4- 5	-804 / 541	-77.3	-77.3	0.14 (8)	6.25	4-11	-145 / 147	0.06 (5)
5- 6	-988 / 558	-77.3	-77.3	0.39 (8)	6.04	11- 5	-163 / 290	0.21 (7)
6- 7	-1149 / 515	-77.3	-77.3	0.39 (8)	5.68	11- 6	-321 / 336	0.26 (4)
14- 2	-1211 / 560	0.0	0.0	0.12 (1)	7.32	9- 6	-184 / 166	0.07 (3)
8- 7	-1091 / 492	0.0	0.0	0.11 (1)	7.60	2-13	-322 / 1064	0.23 (1)
						9- 7	-330 / 1010	0.22 (1)
14-13	-338 / 329	-17.5	-17.5	0.12 (11)	6.25			
13-12	-508 / 1125	-17.5	-17.5	0.22 (1)	6.25			
12-11	-224 / 828	-17.5	-17.5	0.15 (1)	6.25			
11-10	-290 / 971	-17.5	-17.5	0.20 (1)	6.25			
10- 9	-290 / 971	-17.5	-17.5	0.20 (1)	6.25			
9- 8	-13 / 28	-17.5	-17.5	0.10 (11)	6.25			

WIND LOADS APPLIED IS BASED ON REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX ABOVE THE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, GCp, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}. INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE OCCUPIED {OPEN} FROM AT LEAST {0-0} FT-IN-SX AWAY

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.75")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.75")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.43 (2-3:7) , BC=0.22 (12-13:1) , WB=0.34 (3-12:3) , SSI=0.17 (2-3:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (7) (INPUT = 0.90)
JSI METAL= 0.43 (2) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
T.L. WISE
100083566
PROVINCE OF ONTARIO
March 10, 2017

KOTT

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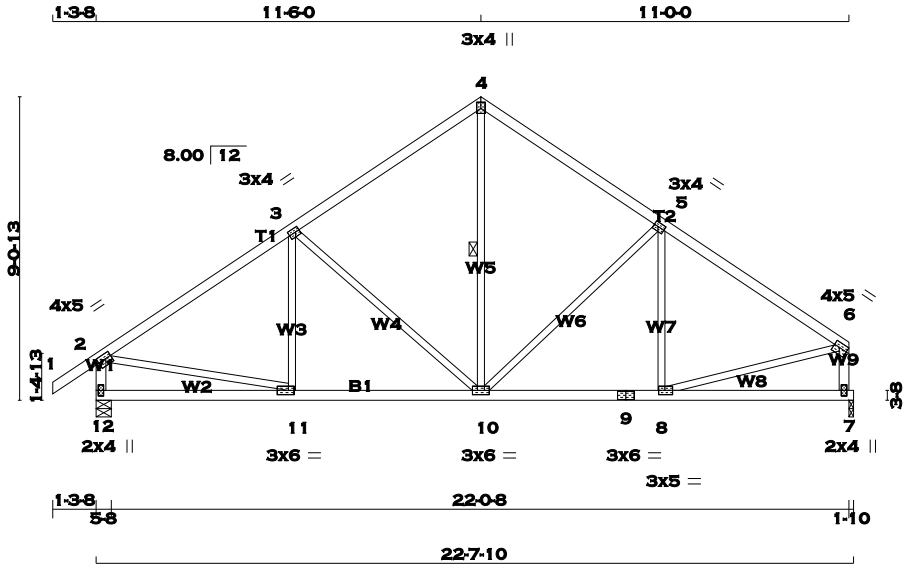
17-4978
BUILDING DIVISION



TOTAL WEIGHT = 97 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 6	2x4	DRY	No.2	SPF
12 - 2	2x4	DRY	No.2	SPF
7 - 6	2x4	DRY	No.2	SPF
12 - 9	2x4	DRY	No.2	SPF
9 - 7	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-t	MT20	4.0	5.0	1.75	2.00
3	TMWW-t	MT20	3.0	4.0	1.50	1.50
4	TTW+p	MT20	3.0	4.0	2.25	1.50
5	TMWW-t	MT20	3.0	4.0	1.50	1.50
6	TMVW-t	MT20	4.0	5.0	1.75	Edge
7	BMV1+p	MT20	2.0	4.0		
8	BMWW-t	MT20	3.0	5.0	1.50	2.25
9	BS-t	MT20	3.0	6.0		
10	BMWWW-t	MT20	3.0	6.0		
11	BMWW-t	MT20	3.0	6.0	1.50	2.00
12	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
12	1173	0	1247	394	-511	5-8	5-8
7	1067	0	1132	0	-444	1-10	1-10

PROVIDE ANCHORAGE AT BEARING JOINT 12 FOR 511 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 7 FOR 444 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 394 LBS FACTORED HORIZONTAL REACTION AT JOINT 12

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
12	821	588 / 0	0 / 0	0 / 0	183 / -515	233 / 0	0 / 0
7	749	524 / 0	0 / 0	0 / 0	163 / -462	225 / 0	0 / 0
HORIZONTAL REACTIONS							
12	---	0 / 0	0 / 0	0 / 0	281 / -267	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 12, 7

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.36 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 4-10. DBS = 20-0-0 . CBF = 40 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 29	-77.3	-77.3	0.10 (1)	10.00	11- 3	-83 / 140	0.03 (4)
2- 3	-1243 / 519	-77.3	-77.3	0.52 (7)	5.36	3-10	-501 / 458	0.56 (3)
3- 4	-929 / 532	-77.3	-77.3	0.52 (7)	6.03	10- 4	-370 / 647	0.19 (7)
4- 5	-929 / 540	-77.3	-77.3	0.48 (8)	6.09	10- 5	-422 / 414	0.46 (4)
5- 6	-1161 / 484	-77.3	-77.3	0.48 (8)	5.55	8- 5	-143 / 158	0.06 (3)
12- 2	-1204 / 539	0.0	0.0	0.12 (1)	7.33	2-11	-306 / 1074	0.23 (1)
7- 6	-1091 / 471	0.0	0.0	0.11 (1)	7.60	8- 6	-317 / 1017	0.22 (1)
12-11	-373 / 364	-17.5	-17.5	0.14 (11)	6.25			
11-10	-488 / 1129	-17.5	-17.5	0.23 (1)	6.25			
10- 9	-280 / 986	-17.5	-17.5	0.21 (1)	6.25			
9- 8	-280 / 986	-17.5	-17.5	0.21 (1)	6.25			
8- 7	-13 / 28	-17.5	-17.5	0.13 (11)	6.25			

WIND LOAD APPLIED IS BASED ON A REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0 } FT IN-SX HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM} INTERNAL WIND PRESSURE IS ASSUMED DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0.0} FT IN-SX AWAY FROM EAVE.

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	23.3	PSF
		DL =	3.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.0	PSF
TOTAL	LOAD	=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.75")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.75")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.52 (2-3:7) , BC=0.23 (10-11:1) , WB=0.56 (3-10:3) , SSI=0.19 (2-3:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (8) (INPUT = 0.90)
JSI METAL= 0.43 (2) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
T.L. WISE
100083566
PROVINCE OF ONTARIO

March 10, 2017

KOTT

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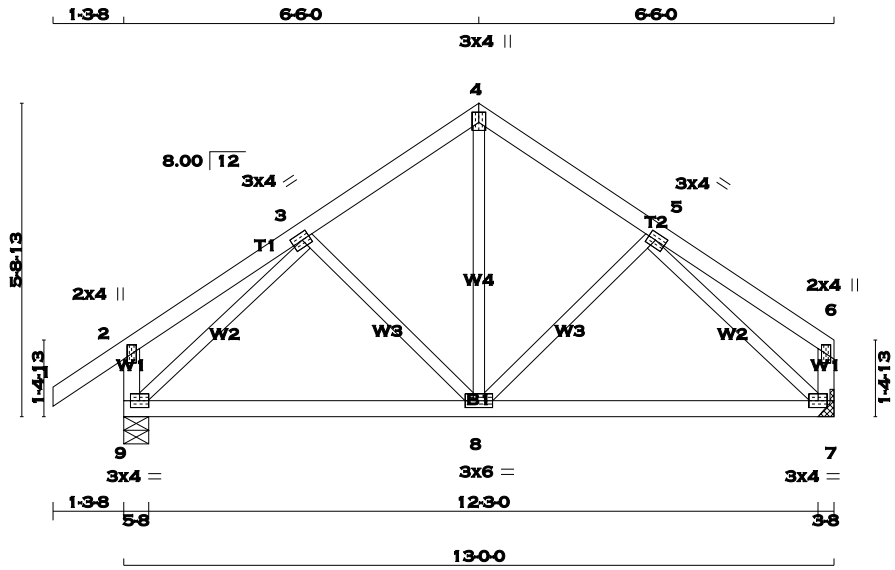
BUILDING DIVISION



TOTAL WEIGHT = 2 X 55 = 110 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 4	2x4	DRY	No.2	SPF
4 - 6	2x4	DRY	No.2	SPF
9 - 2	2x4	DRY	No.2	SPF
7 - 6	2x4	DRY	No.2	SPF
9 - 7	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMV+p	MT20	2.0	4.0		
3	TMWW-t	MT20	3.0	4.0	1.50	1.75
4	TTW+p	MT20	3.0	4.0	2.25	1.50
5	TMWW-t	MT20	3.0	4.0	1.50	1.75
6	TMV+p	MT20	2.0	4.0		
7	BMVW1-t	MT20	3.0	4.0		
8	BMWWW-t	MT20	3.0	6.0		
9	BMVW1-t	MT20	3.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
9	723	0	764	256	-315	5-8	5-8
7	617	0	653	0	-256	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

PROVIDE ANCHORAGE AT BEARING JOINT 9 FOR 315 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 7 FOR 256 LBS FACTORED UPLIFT

PROVIDE FOR 256 LBS FACTORED HORIZONTAL REACTION AT JOINT 9

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
9	505	367 / 0	0 / 0	0 / 0	104 / -314	138 / 0	0 / 0
7	433	303 / 0	0 / 0	0 / 0	91 / -267	130 / 0	0 / 0
HORIZONTAL REACTIONS							
9	---	0 / 0	0 / 0	0 / 0	183 / -174	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 9

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 29	-77.3	-77.3	0.10 (1)	10.00	8- 4	-193 / 344	0.09 (7)
2- 3	-24 / 151	-77.3	-77.3	0.16 (7)	6.25	8- 5	-175 / 272	0.06 (4)
3- 4	-518 / 294	-77.3	-77.3	0.18 (7)	6.25	3- 8	-175 / 283	0.06 (3)
4- 5	-519 / 297	-77.3	-77.3	0.18 (8)	6.25	9- 3	-718 / 221	0.23 (4)
5- 6	-20 / 94	-77.3	-77.3	0.15 (8)	6.25	5- 7	-711 / 251	0.22 (3)
9- 2	-235 / 225	0.0	0.0	0.04 (7)	7.81			
7- 6	-122 / 129	0.0	0.0	0.02 (8)	7.81			
9- 8	-260 / 546	-17.5	-17.5	0.24 (11)	6.25			
8- 7	-156 / 503	-17.5	-17.5	0.24 (11)	6.25			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.18 (3-4:7) , BC=0.24 (8-9:11) , WB=0.23 (3-9:4) , SSI=0.11 (4-5:8)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

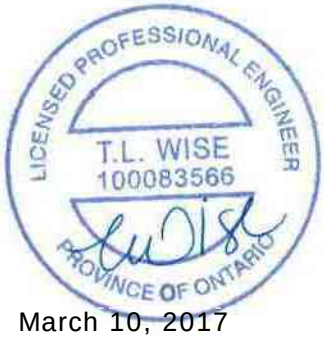
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (7) (INPUT = 0.90)
JSI METAL= 0.26 (5) (INPUT = 1.00)



March 10, 2017

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

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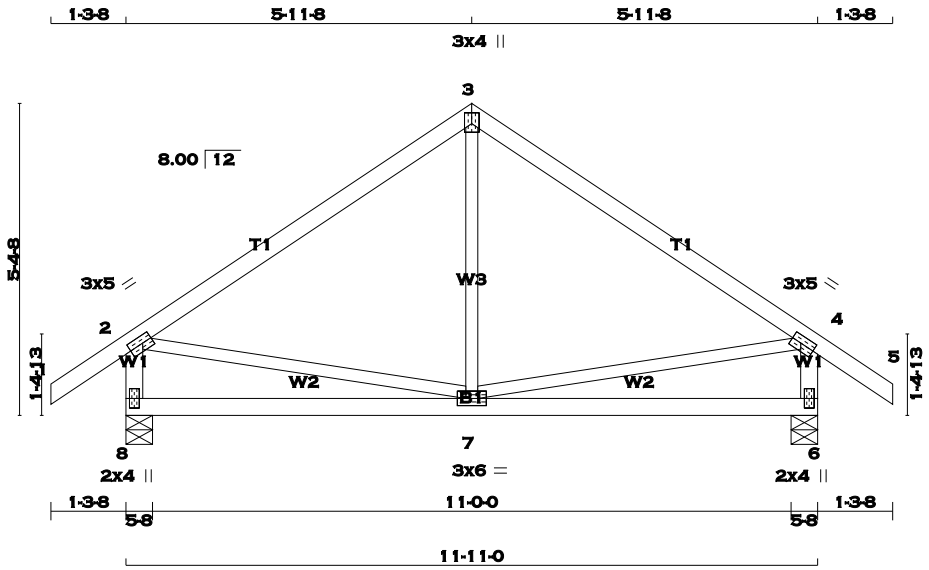


TOTAL WEIGHT = 2 X 49 = 98 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 3	2x4	DRY	No.2	SPF
3 - 5	2x4	DRY	No.2	SPF
8 - 2	2x4	DRY	No.2	SPF
6 - 4	2x4	DRY	No.2	SPF
8 - 6	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
2	TMVW-t	MT20	3.0	5.0	1.50	2.00
3	TTW+p	MT20	3.0	4.0	2.25	1.50
4	TMVW-t	MT20	3.0	5.0	1.50	2.00
6	BMV1+p	MT20	2.0	4.0		
7	BMWWW-t	MT20	3.0	6.0		
8	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
8	671	0	709	-254	-295	5-8	5-8
6	671	0	709	0	-295	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT 8 FOR 295 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 6 FOR 295 LBS FACTORED UPLIFT

PROVIDE FOR 254 LBS FACTORED HORIZONTAL REACTION AT JOINT 8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
8	469	341 / 0	0 / 0	0 / 0	94 / -292	127 / 0	0 / 0
6	469	341 / 0	0 / 0	0 / 0	94 / -292	127 / 0	0 / 0
HORIZONTAL REACTIONS							
8	---	0 / 0	0 / 0	0 / 0	181 / -181	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 8, 6

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	0 / 29	-77.3	-77.3	0.10 (1)	10.00	7- 3	0 / 104	0.04 (11)
2- 3	-455 / 216	-77.3	-77.3	0.51 (7)	6.25	2- 7	-73 / 393	0.08 (1)
3- 4	-455 / 216	-77.3	-77.3	0.51 (8)	6.25	7- 4	-73 / 393	0.08 (1)
4- 5	0 / 29	-77.3	-77.3	0.10 (1)	10.00			
8- 2	-669 / 323	0.0	0.0	0.07 (1)	7.81			
6- 4	-669 / 323	0.0	0.0	0.07 (1)	7.81			
8- 7	-233 / 244	-17.5	-17.5	0.18 (11)	6.25			
7- 6	-10 / 21	-17.5	-17.5	0.18 (11)	10.00			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	23.3	PSF
		DL =	3.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.0	PSF
TOTAL	LOAD	=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.40")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.40")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.51 (3-4:8) , BC=0.18 (7-8:11) , WB=0.08 (2-7:1) , SSI=0.16 (3-4:8)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

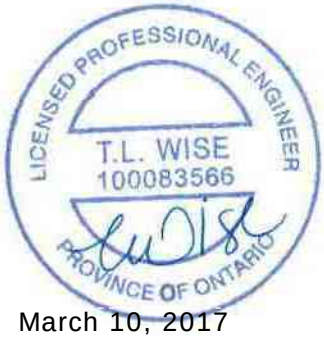
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (7) (INPUT = 0.90)
JSI METAL= 0.19 (2) (INPUT = 1.00)



March 10, 2017

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

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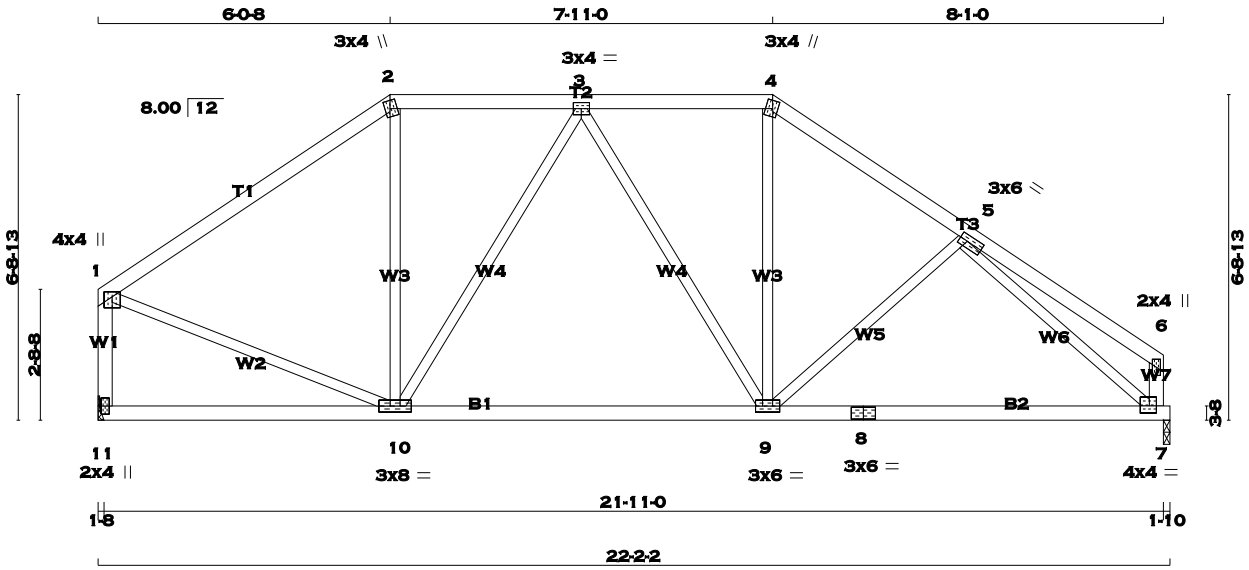

TOTAL WEIGHT = 93 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 2	2x4	DRY	No.2	SPF
2 - 4	2x4	DRY	No.2	SPF
4 - 6	2x4	DRY	No.2	SPF
11 - 1	2x4	DRY	No.2	SPF
7 - 6	2x4	DRY	No.2	SPF
11 - 8	2x4	DRY	No.2	SPF
8 - 7	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
1	TMVW+p	MT20	4.0	4.0	1.25	2.00
2	TTW+m	MT20	3.0	4.0	2.00	1.25
3	TMWW-t	MT20	3.0	4.0		
4	TTW+m	MT20	3.0	4.0	2.00	1.25
5	TMWW-t	MT20	3.0	6.0	1.50	2.00
6	TMV+p	MT20	2.0	4.0		
7	BMVW1-t	MT20	4.0	4.0	1.75	1.75
8	BS-t	MT20	3.0	6.0		
9	BMWWW-t	MT20	3.0	6.0		
10	BMWWW-t	MT20	3.0	8.0		
11	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
11	1045	0	1105	-300	-492	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
7	1045	0	1098	0	-505	1-10	1-10

PROVIDE ANCHORAGE AT BEARING JOINT 11 FOR 492 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 7 FOR 505 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 300 LBS FACTORED HORIZONTAL REACTION AT JOINT 11

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
11	734	513 / 0	0 / 0	0 / 0	149 / -493	220 / 0	0 / 0
7	734	513 / 0	0 / 0	0 / 0	131 / -503	220 / 0	0 / 0
HORIZONTAL REACTIONS							
11	---	0 / 0	0 / 0	0 / 0	191 / -214	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 7

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	-926 / 466	-77.3	-77.3	0.57 (7)	5.93	10- 2	-53 / 149	0.04 (8)
2- 3	-792 / 519	-77.3	-77.3	0.21 (7)	6.25	10- 3	-362 / 285	0.40 (4)
3- 4	-899 / 584	-77.3	-77.3	0.22 (8)	6.25	3- 9	-131 / 198	0.15 (3)
4- 5	-1087 / 614	-77.3	-77.3	0.30 (8)	5.91	9- 4	-154 / 347	0.11 (7)
5- 6	-21 / 109	-77.3	-77.3	0.23 (8)	6.25	9- 5	-186 / 309	0.09 (4)
11- 1	-1063 / 521	0.0	0.0	0.14 (1)	7.67	1-10	-275 / 800	0.20 (8)
7- 6	-149 / 156	0.0	0.0	0.03 (8)	7.81	5- 7	-1325 / 576	0.63 (1)
11-10	-221 / 278	-17.5	-17.5	0.17 (11)	6.25			
10- 9	-335 / 963	-17.5	-17.5	0.34 (11)	6.25			
9- 8	-404 / 975	-17.5	-17.5	0.34 (11)	6.25			
8- 7	-404 / 975	-17.5	-17.5	0.34 (11)	6.25			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.57 (1-2:7) , BC=0.34 (9-10:11) , WB=0.63 (5-7:1) , SSI=0.16 (1-2:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (9) (INPUT = 0.90)
JSI METAL= 0.37 (7) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
T.L. WISE
100083566
PROVINCE OF ONTARIO

March 10, 2017

KOTT

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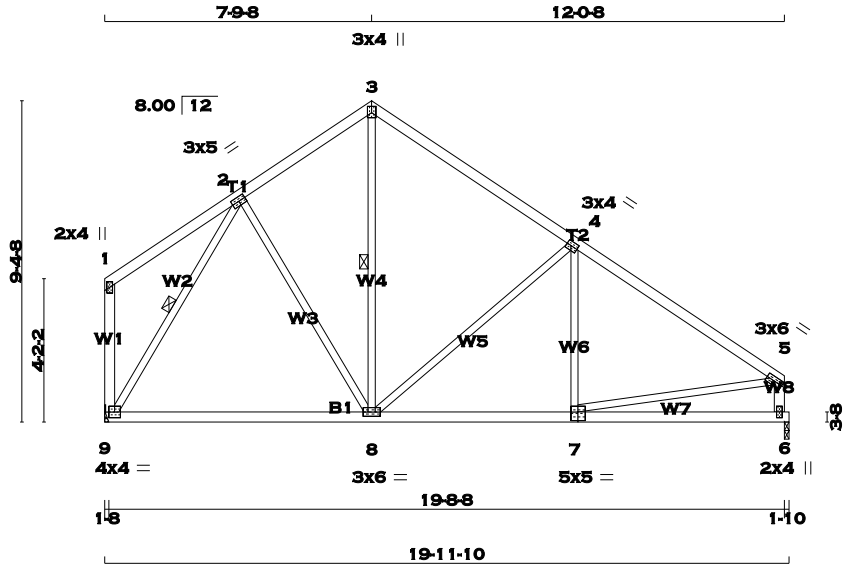

SCALE = 1:67.2

TOTAL WEIGHT = 4 X 89 = 357 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 3	2x4	DRY	No.2	SPF
3 - 5	2x4	DRY	No.2	SPF
9 - 1	2x4	DRY	No.2	SPF
6 - 5	2x4	DRY	No.2	SPF
9 - 7	2x4	DRY	No.2	SPF
7 - 6	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
1	TMV+p	MT20	2.0	4.0		
2	TMWW-t	MT20	3.0	5.0		
3	TTW+p	MT20	3.0	4.0	2.25	1.50
4	TMWW-t	MT20	3.0	4.0	1.50	1.50
5	TMVW-t	MT20	3.0	6.0		
6	BMV1+p	MT20	2.0	4.0		
7	BSWW-l	MT20	5.0	5.0	3.00	2.50
8	BMWWW-t	MT20	3.0	6.0		
9	BMVW1-t	MT20	4.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
9	941	0	997	-433	-387	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
6	941	0	1004	0	-402	1-10	1-10

PROVIDE ANCHORAGE AT BEARING JOINT 9 FOR 387 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 6 FOR 402 LBS FACTORED UPLIFT

PROVIDE FOR 433 LBS FACTORED HORIZONTAL REACTION AT JOINT 9

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
9	660	462 / 0	0 / 0	0 / 0	142 / -404	198 / 0	0 / 0
6	660	462 / 0	0 / 0	0 / 0	159 / -414	198 / 0	0 / 0
HORIZONTAL REACTIONS							
9	---	0 / 0	0 / 0	0 / 0	262 / -310	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 6

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.63 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 3-8. DBS = 20-0-0 . CBF = 32 LBS.
1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 2-9. DBS = 16-0-0 . CBF = 90 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	-49 / 174	-77.3	-77.3	0.23 (7)	6.25	2- 8	-79 / 271	0.09 (5)
2- 3	-682 / 469	-77.3	-77.3	0.26 (7)	6.25	8- 3	-298 / 430	0.16 (7)
3- 4	-696 / 424	-77.3	-77.3	0.55 (8)	6.25	8- 4	-561 / 499	0.69 (4)
4- 5	-1060 / 446	-77.3	-77.3	0.55 (8)	5.63	7- 4	-46 / 120	0.03 (11)
9- 1	-161 / 189	0.0	0.0	0.11 (7)	7.81	9- 2	-965 / 343	0.34 (4)
6- 5	-958 / 433	0.0	0.0	0.09 (1)	7.81	7- 5	-255 / 913	0.20 (1)
9- 8	-243 / 553	-17.5	-17.5	0.26 (11)	6.25			
8- 7	-231 / 901	-17.5	-17.5	0.29 (11)	6.25			
7- 6	-9 / 20	-17.5	-17.5	0.14 (11)	10.00			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPE... TERRAIN ... TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FRO... EA...

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	23.3	PSF
		DL =	3.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.0	PSF
TOTAL	LOAD	=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.55 (4-5:8) , BC=0.29 (7-8:11) , WB=0.69 (4-8:4) , SSI=0.20 (4-5:8)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (2) (INPUT = 0.90)
JSI METAL= 0.30 (5) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
T.L. WISE
100083566
PROVINCE OF ONTARIO

March 10, 2017

KOTT

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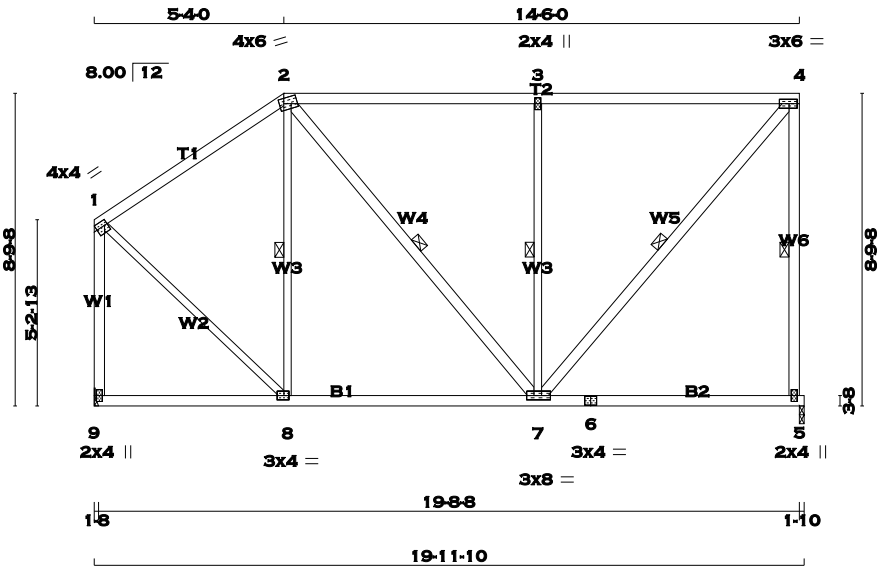


SCALE = 1:64.8

TOTAL WEIGHT = 105 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 2	2x4	DRY	No.2	SPF
2 - 4	2x4	DRY	No.2	SPF
5 - 4	2x4	DRY	No.2	SPF
9 - 1	2x4	DRY	No.2	SPF
9 - 6	2x4	DRY	No.2	SPF
6 - 5	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
2 - 7	2x4	DRY	No.2	SPF
7 - 4	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
1	TMVW-t	MT20	4.0	4.0	1.75	Edge
2	TTWW-m	MT20	4.0	6.0	1.75	1.50
3	TMW+w	MT20	2.0	4.0		
4	TMVW-t	MT20	3.0	6.0	1.50	2.75
5	BMV1+p	MT20	2.0	4.0		
6	BS-t	MT20	3.0	4.0		
7	BMWWW-t	MT20	3.0	8.0	1.50	2.50
8	BMWW-t	MT20	3.0	4.0	1.50	1.75
9	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
5	941	0	1007	0	-531	1-10	1-10
9	941	0	989	465	-461	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

PROVIDE ANCHORAGE AT BEARING JOINT 5 FOR 531 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 9 FOR 461 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 465 LBS FACTORED HORIZONTAL REACTION AT JOINT 9

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
5	660	462 / 0	0 / 0	0 / 0	165 / -507	198 / 0	0 / 0
9	660	462 / 0	0 / 0	0 / 0	121 / -457	198 / 0	0 / 0
HORIZONTAL REACTIONS							
9	---	0 / 0	0 / 0	0 / 0	332 / -280	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 5

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.03 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 4-5. DBS = 16-0-0 . CBF = 89 LBS.
1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 2-8, 2-7, 3-7, 4-7. DBS = 20-0-0 . CBF = 87 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	-590 / 396	-77.3	-77.3	0.45 (7)	6.25	8- 2	-296 / 189	0.14 (4)
2- 3	-632 / 505	-77.3	-77.3	0.73 (1)	6.03	2- 7	-154 / 253	0.10 (8)
3- 4	-632 / 506	-77.3	-77.3	0.73 (1)	6.03	7- 3	-734 / 565	0.35 (3)
5- 4	-955 / 567	0.0	0.0	0.50 (7)	6.25	7- 4	-518 / 961	0.34 (7)
9- 1	-953 / 487	0.0	0.0	0.46 (1)	7.81	1- 8	-160 / 615	0.16 (8)
9- 8	-369 / 347	-17.5	-17.5	0.14 (11)	6.25			
8- 7	-345 / 500	-17.5	-17.5	0.26 (11)	6.25			
7- 6	-65 / 167	-17.5	-17.5	0.23 (11)	6.25			
6- 5	-65 / 167	-17.5	-17.5	0.23 (11)	6.25			

WIND LOAD APPLIED TO THIS TRUSS HAS BEEN DETERMINED FROM A REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-SX ABOVE GRADE IN {OPEN} TERRAIN. THE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, AND BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE COEFFICIENTS ARE BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN} TERRAIN, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.73 (3-4:1) , BC=0.26 (7-8:11) , WB=0.35 (3-7:3) , SSI=0.28 (3-4:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

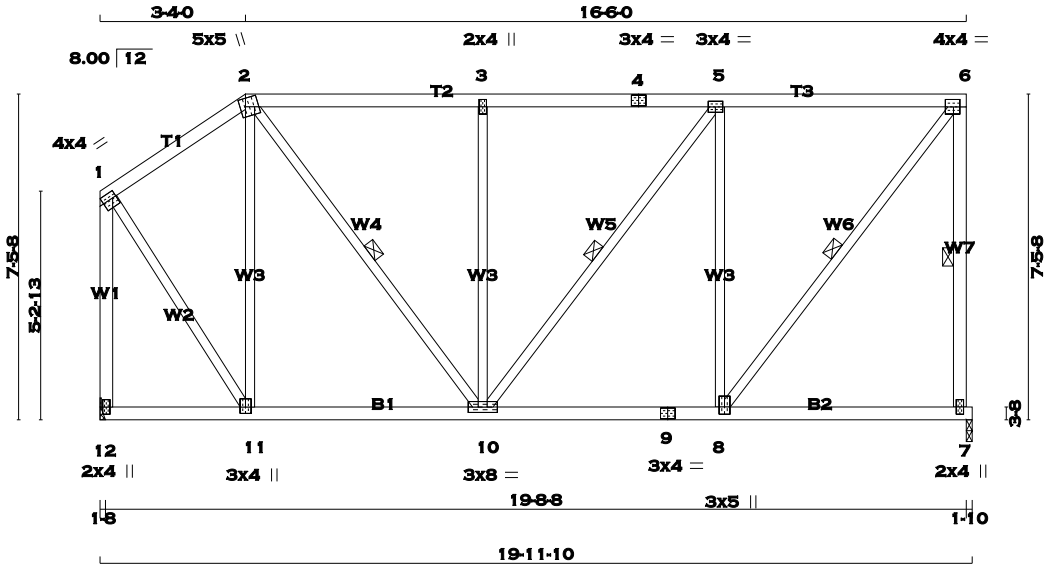
JSI GRIP= 0.90 (8) (INPUT = 0.90)
JSI METAL= 0.26 (1) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
T.L. WISE
100083566
PROVINCE OF ONTARIO

March 10, 2017

KOTT

RECEIVED
TOWN OF MILTON
MAR 29, 2017
17-4978
BUILDING DIVISION



TOTAL WEIGHT = 102 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 2	2x4	DRY	No.2	SPF
2 - 4	2x4	DRY	No.2	SPF
4 - 6	2x4	DRY	No.2	SPF
7 - 6	2x4	DRY	No.2	SPF
12 - 1	2x4	DRY	No.2	SPF
12 - 9	2x4	DRY	No.2	SPF
9 - 7	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
1	TMVW-t	MT20	4.0	4.0	1.75	Edge
2	TTWW+m	MT20	5.0	5.0	2.25	1.50
3	TMW+w	MT20	2.0	4.0		
4	TS-t	MT20	3.0	4.0		
5	TMWW-t	MT20	3.0	4.0		
6	TMVW-t	MT20	4.0	4.0	2.00	1.75
7	BMV1+p	MT20	2.0	4.0		
8	BMWW+t	MT20	3.0	5.0	2.00	1.50
9	BS-t	MT20	3.0	4.0		
10	BMWWW-t	MT20	3.0	8.0		
11	BMWW+t	MT20	3.0	4.0	1.75	1.50
12	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
7	941	0	999	0	-516	1-10	1-10
12	941	0	984	390	-476	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

PROVIDE ANCHORAGE AT BEARING JOINT 7 FOR 516 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 12 FOR 476 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 390 LBS FACTORED HORIZONTAL REACTION AT JOINT 12

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
7	660	462 / 0	0 / 0	0 / 0	145 / -496	198 / 0	0 / 0
12	660	462 / 0	0 / 0	0 / 0	109 / -468	198 / 0	0 / 0
HORIZONTAL REACTIONS							
12	---	0 / 0	0 / 0	0 / 0	279 / -247	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 7

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 6-7. DBS = 16-0-0 . CBF = 90 LBS.
1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 2-10, 5-10, 6-8. DBS = 20-0-0 . CBF = 58 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	-512 / 363	-77.3	-77.3	0.20 (7)	6.25	11- 2	-457 / 245	0.45 (1)
2- 3	-736 / 529	-77.3	-77.3	0.31 (1)	6.25	2-10	-287 / 523	0.15 (8)
3- 4	-736 / 529	-77.3	-77.3	0.34 (1)	6.25	10- 3	-478 / 368	0.47 (3)
4- 5	-736 / 529	-77.3	-77.3	0.34 (1)	6.25	10- 5	-140 / 206	0.08 (6)
5- 6	-611 / 457	-77.3	-77.3	0.34 (1)	6.25	8- 5	-663 / 480	0.65 (3)
7- 6	-959 / 544	0.0	0.0	0.37 (7)	6.25	8- 6	-511 / 984	0.28 (7)
12- 1	-965 / 491	0.0	0.0	0.47 (1)	7.81	1-11	-226 / 666	0.15 (1)
12-11	-294 / 301	-17.5	-17.5	0.08 (11)	6.25			
11-10	-312 / 425	-17.5	-17.5	0.12 (11)	6.25			
10- 9	-266 / 606	-17.5	-17.5	0.17 (11)	6.25			
9- 8	-266 / 606	-17.5	-17.5	0.17 (11)	6.25			
8- 7	-5[illegible] / 14			[illegible])	6.25			

WIND LOAD APPLIED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} MEAN ROOF HEIGHT IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM} INTERNAL WIN[illegible] MAY BE LOCATED ON {OPE[illegible] LEAST {1.0} FT-IN-SX AWAY FRO[illegible]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.47 (1-12:1) , BC=0.17 (8-10:11) , WB=0.65 (5-8:3) , SSI=0.20 (5-6:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (8) (INPUT = 0.90)
JSI METAL= 0.26 (8) (INPUT = 1.00)

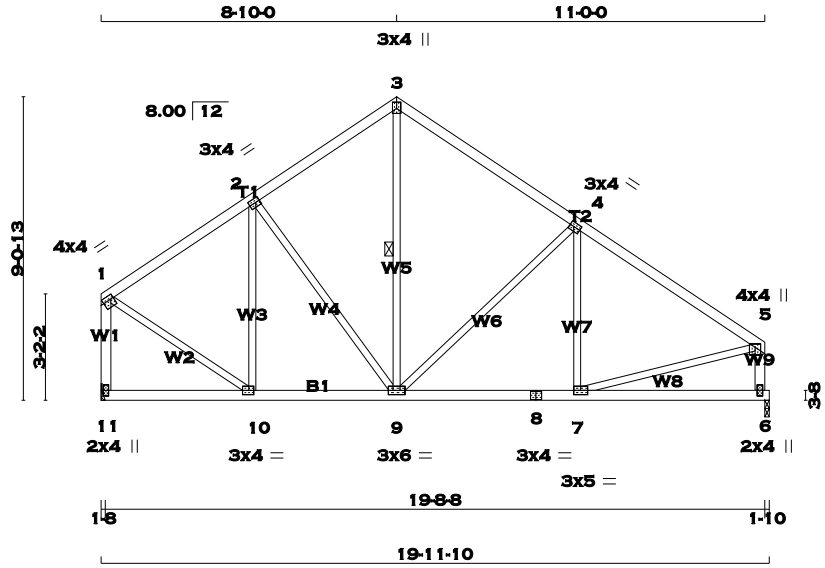
LICENSED PROFESSIONAL ENGINEER
T.L. WISE
100083566
PROVINCE OF ONTARIO

March 10, 2017

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

RECEIVED
TOWN OF AURORA
MAR 20, 2017
17-4978
BUILDING DIVISION



TOTAL WEIGHT = 3 X 90 = 271 lb

[M][F]

LUMBER

N. L. G. A. RULES

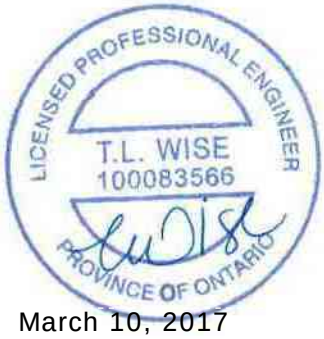
CHORDS	SIZE		LUMBER	DESCR.
1 - 3	2x4	DRY	No.2	SPF
3 - 5	2x4	DRY	No.2	SPF
11 - 1	2x4	DRY	No.2	SPF
6 - 5	2x4	DRY	No.2	SPF
11 - 8	2x4	DRY	No.2	SPF
8 - 6	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
1	TMVW-t	MT20	4.0	4.0	1.50	1.00
2	TMWW-t	MT20	3.0	4.0	1.50	1.50
3	TTW+p	MT20	3.0	4.0	2.25	1.50
4	TMWW-t	MT20	3.0	4.0	1.50	1.50
5	TMVW+p	MT20	4.0	4.0	1.25	2.00
6	BMV1+p	MT20	2.0	4.0		
7	BMWW-t	MT20	3.0	5.0		
8	BS-t	MT20	3.0	4.0		
9	BMWWW-t	MT20	3.0	6.0		
10	BMWW-t	MT20	3.0	4.0	1.50	1.75
11	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.



March 10, 2017

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
11	941	0	989	-399	-372	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
6	941	0	1001	0	-395	1-10	1-10

PROVIDE ANCHORAGE AT BEARING JOINT 11 FOR 372 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 6 FOR 395 LBS FACTORED UPLIFT

PROVIDE FOR 399 LBS FACTORED HORIZONTAL REACTION AT JOINT 11

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
11	660	462 / 0	0 / 0	0 / 0	121 / -393	198 / 0	0 / 0
6	660	462 / 0	0 / 0	0 / 0	150 / -410	198 / 0	0 / 0
HORIZONTAL REACTIONS							
11	---	0 / 0	0 / 0	0 / 0	261 / -285	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 6

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x3 SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 3-9. DBS = 20-0-0 . CBF = 35 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
1- 2	-740 / 364	-77.3	-77.3	0.31 (7)	6.25
2- 3	-721 / 476	-77.3	-77.3	0.32 (7)	6.25
3- 4	-727 / 452	-77.3	-77.3	0.47 (8)	6.25
4- 5	-990 / 420	-77.3	-77.3	0.47 (8)	5.89
11- 1	-956 / 394	0.0	0.0	0.16 (1)	7.81
6- 5	-960 / 422	0.0	0.0	0.10 (1)	7.81
11-10	-309 / 373	-17.5	-17.5	0.08 (5)	6.25
10- 9	-256 / 689	-17.5	-17.5	0.14 (1)	6.25
9- 8	-216 / 842	-17.5	-17.5	0.20 (1)	6.25
8- 7	-216 / 842	-17.5	-17.5	0.20 (1)	6.25
7- 6	-13 / 28	-17.5	-17.5	0.13 (11)	6.25

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
10- 2	-316 / 187	0.19 (4)
2- 9	-177 / 293	0.18 (3)
9- 3	-319 / 469	0.16 (7)
9- 4	-456 / 425	0.49 (4)
7- 4	-104 / 143	0.04 (3)
1-10	-233 / 750	0.16 (1)
7- 5	-251 / 869	0.19 (1)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURES ARE BASED ON {ENCLOSURE CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN AND TOPOGRAPHIC CLASS 1}. TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.47 (3-4:8) , BC=0.20 (7-9:1) , WB=0.49 (4-9:4) , SSI=0.18 (4-5:8)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

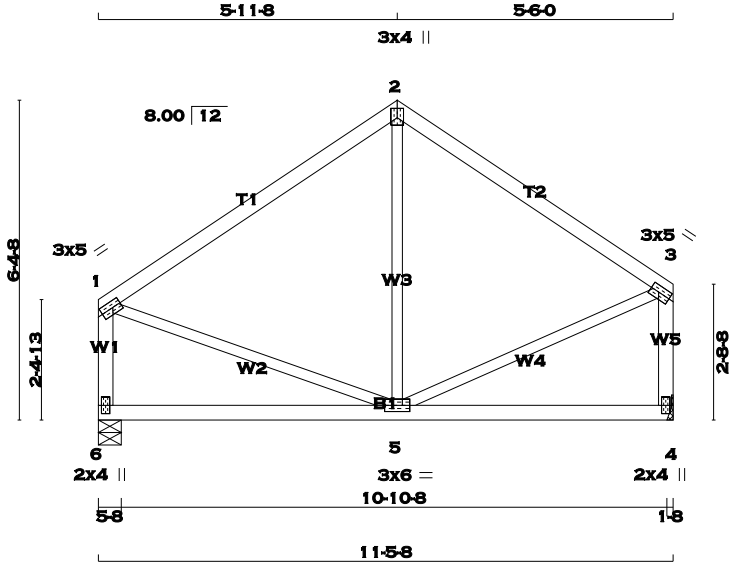
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (10) (INPUT = 0.90)
JSI METAL= 0.31 (1) (INPUT = 1.00)



SCALE = 1:45.9

TOTAL WEIGHT = 2 X 48 = 96 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 2	2x4	DRY	No.2	SPF
2 - 3	2x4	DRY	No.2	SPF
6 - 1	2x4	DRY	No.2	SPF
4 - 3	2x4	DRY	No.2	SPF
6 - 4	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
1	TMVW-t	MT20	3.0	5.0	1.50	2.00
2	TTW+p	MT20	3.0	4.0	2.25	1.50
3	TMVW-t	MT20	3.0	5.0	1.50	2.00
4	BMV1+p	MT20	2.0	4.0		
5	BMWWW-t	MT20	3.0	6.0		
6	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
6	543	0	573	282	-215	5-8	5-8
4	543	0	575	0	-214	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

PROVIDE ANCHORAGE AT BEARING JOINT 6 FOR 215 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 4 FOR 214 LBS FACTORED UPLIFT

PROVIDE FOR 282 LBS FACTORED HORIZONTAL REACTION AT JOINT 6

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
6	381	267 / 0	0 / 0	0 / 0	74 / -227	115 / 0	0 / 0
4	381	267 / 0	0 / 0	0 / 0	80 / -227	115 / 0	0 / 0
HORIZONTAL REACTIONS							
6	---	0 / 0	0 / 0	0 / 0	201 / -196	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 6

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	-359 / 201	-77.3	-77.3	0.51 (7)	6.25	5- 2	-89 / 92	0.06 (3)
2- 3	-359 / 210	-77.3	-77.3	0.44 (8)	6.25	1- 5	-79 / 318	0.06 (1)
6- 1	-533 / 244	0.0	0.0	0.06 (1)	7.81	5- 3	-84 / 327	0.07 (1)
4- 3	-540 / 239	0.0	0.0	0.07 (1)	7.81			
6- 5	-241 / 256	-17.5	-17.5	0.17 (11)	6.25			
5- 4	-22 / 47	-17.5	-17.5	0.17 (11)	6.25			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.38")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.51 (1-2:7) , BC=0.17 (4-5:11) , WB=0.07 (3-5:1) , SSI=0.16 (1-2:7)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.53 (5) (INPUT = 0.90)
JSI METAL= 0.15 (1) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
T.L. WISE
100083566
PROVINCE OF ONTARIO

March 10, 2017

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

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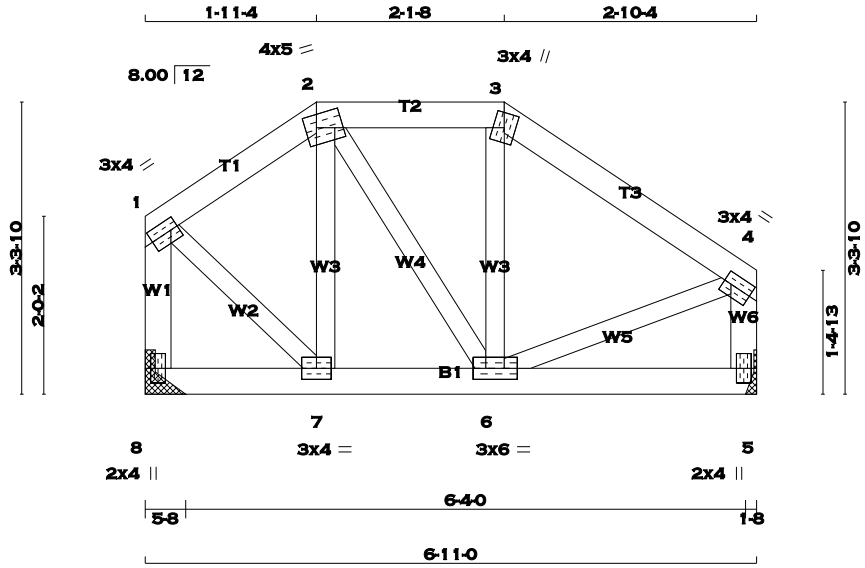

TOTAL WEIGHT = 31 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 2	2x4	DRY	No.2	SPF
2 - 3	2x4	DRY	No.2	SPF
3 - 4	2x4	DRY	No.2	SPF
8 - 1	2x4	DRY	No.2	SPF
5 - 4	2x4	DRY	No.2	SPF
8 - 5	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
1	TMVW-t	MT20	3.0	4.0	1.50	1.00
2	TTWW-m	MT20	4.0	5.0	1.75	1.50
3	TTW+m	MT20	3.0	4.0		
4	TMVW-t	MT20	3.0	4.0	1.50	1.00
5	BMV1+p	MT20	2.0	4.0		
6	BMWWW-t	MT20	3.0	6.0		
7	BMWW-t	MT20	3.0	4.0		
8	BMV1+p	MT20	2.0	4.0		

A SIZE FOR SIZE SUBSTITUTION OF MITEK MII20 WITH TEE-LOK TL20 PLATES IS ALLOWED.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
8	328	0	341	-150	-146	HANGER BY OTHERS MIN. SEAT SIZE: 5-8	
5	328	0	342	0	-151	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

PROVIDE ANCHORAGE AT BEARING JOINT 8 FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 5 FOR 151 LBS FACTORED UPLIFT

PROVIDE FOR 150 LBS FACTORED HORIZONTAL REACTION AT JOINT 8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
8	230	161 / 0	0 / 0	0 / 0	33 / -149	69 / 0	0 / 0
5	230	161 / 0	0 / 0	0 / 0	36 / -152	69 / 0	0 / 0
HORIZONTAL REACTIONS							
8	---	0 / 0	0 / 0	0 / 0	97 / -107	0 / 0	0 /0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (11)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
1- 2	-207 / 137	-77.3	-77.3	0.07 (7)	6.25	7- 2	-102 / 65	0.02 (1)
2- 3	-201 / 166	-77.3	-77.3	0.06 (8)	6.25	2- 6	-53 / 71	0.01 (4)
3- 4	-243 / 128	-77.3	-77.3	0.13 (8)	6.25	6- 3	-76 / 86	0.02 (8)
8- 1	-325 / 157	0.0	0.0	0.04 (7)	7.81	1- 7	-69 / 215	0.05 (1)
5- 4	-319 / 165	0.0	0.0	0.03 (1)	7.81	6- 4	-48 / 215	0.04 (1)
8- 7	-103 / 135	-17.5	-17.5	0.03 (5)	6.25			
7- 6	-83 / 177	-17.5	-17.5	0.04 (11)	6.25			
6- 5	-10 / 21	-17.5	-17.5	0.03 (11)	10.00			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.0} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.23")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.13 (3-4:8) , BC=0.04 (6-7:11) , WB=0.05 (1-7:1) , SSI=0.08 (3-4:8)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.65 (4) (INPUT = 0.90)
JSI METAL= 0.11 (4) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
T.L. WISE
100083566
PROVINCE OF ONTARIO

March 10, 2017

KOTT

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

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MAR 29, 2017
17-4978
BUILDING DIVISION

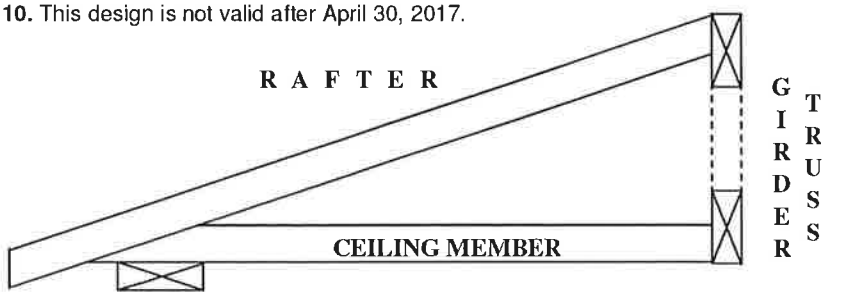
BEARING ANCHORAGE BY TOE-NAILS FOR LATERAL CAPACITY

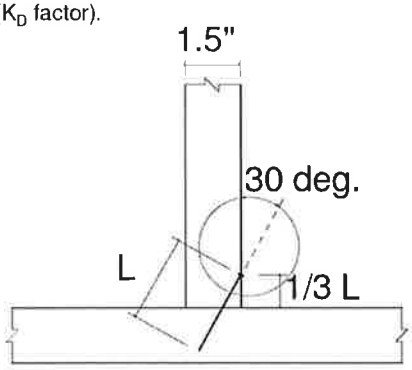
B37579H1

NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL LATERAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	132	147
	3.25	0.144	132	147
	3.50	0.160	159	177
COMMON SPIRAL	3.00	0.122	97	108
	3.25	0.122	97	108
	3.50	0.152	145	162

NOTES:

1. Rafter and ceiling members may be anchored to top and bottom chords of girder truss by toe-nailing rafter and ceiling members to girder chords provided the reaction does not exceed the lateral capacities in the table. Hangers (specified by others) are required for reactions higher than the maximum toe-nail capacity. Reactions are based on factored loads.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-09, section 10.9.4.1.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in tables below.
5. Nail values in table are based on the following relative lumber densities: G = 0.42 (SPF), G = 0.49 (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See next page for nailing on bearing plate).
7. For loads due to **wind** the nail lateral capacity in this table may be multiplied by 1.15 (K_D factor).
8. Lumber must be dry (< 19% moisture content) at the time of nail installation.
9. Nail values in this table comply with CSA O86-09, section 10.9.4
10. This design is not valid after April 30, 2017.





TOE-NAIL INSTALLATION

Nail type	Common wire	Common spiral	Common wire	Common spiral
Nail dia. (in)	0.160	0.152	0.144	0.122
	(3.5" nail)		(3" and 3.25" nail)	
LUMBER SIZE	MAXIMUM NUMBER OF TOE-NAILS			
2X4 SPF	2	2	3	3
2X4 D. Fir	2	2	2	2
2X6 SPF	4	4	4	5
2X6 D. Fir	3	3	3	4

MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

PEO
Certificate No. 10889485

April 24, 2015

BEARING ANCHORAGE BY TOE-NAILS FOR WIND LOADING B37579H2

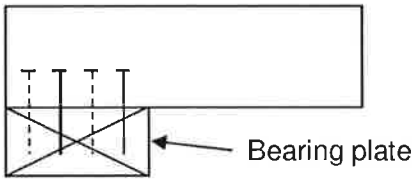
NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL WITHDRAWAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	30	42
	3.25	0.144	32	45
	3.50	0.160	38	52
COMMON SPIRAL	3.00	0.122	26	36
	3.25	0.122	28	40
	3.50	0.152	36	50

Note: If using truss with D. Fir lumber and S-P-F bearing plate, use values in table for S-P-F.

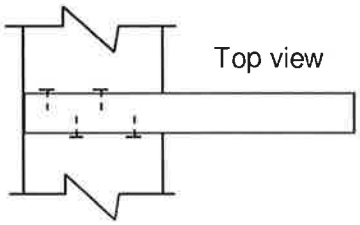
NOTES:

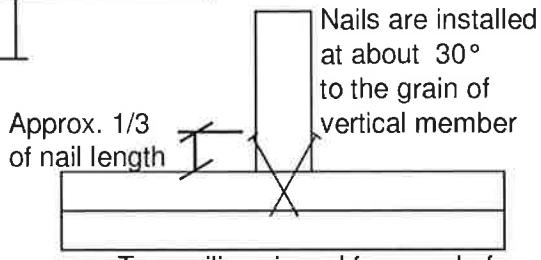
1. Truss chord, rafter, or ceiling members may be anchored to bearing plate by toe-nails, provided that the actual factored uplift force due to **wind** or **earthquake** load does not exceed the withdrawal capacities in the table. Hangers (specified by others) are required for uplift forces that are higher than the maximum toe-nail withdrawal capacity.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-09, section 10.9.5.2.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in table above.
5. Nail values in table are based on the following relative lumber densities: G = 0.42(SPF), G = 0.49(D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See drawing on detail B37579H1).
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-09, section 10.9.5
9. This design is not valid after April 30, 2017

Toe-nailing on 2x6 Bearing Plate



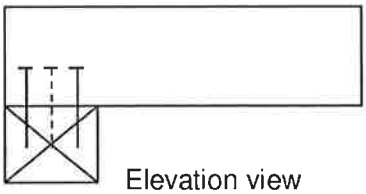
Elevation view



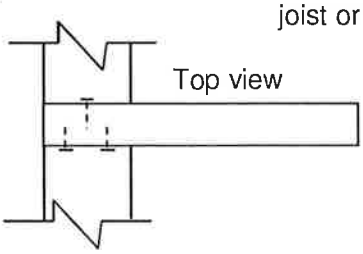


Toe-nailing viewed from end of joist or truss

Toe-nailing on 2x4 Bearing Plate



Elevation view



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