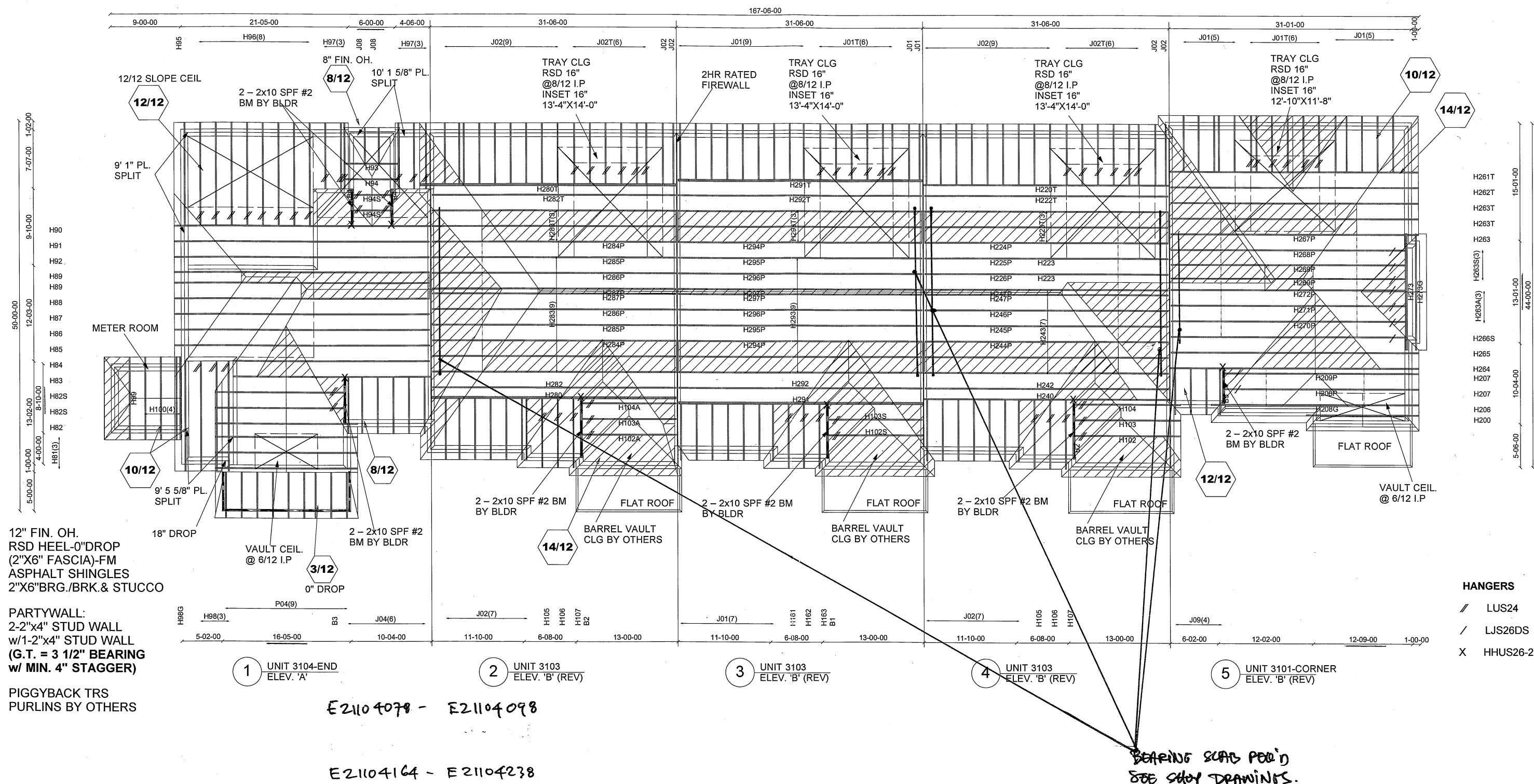




L: 111206(TYP.), 114170E

ALL CONVENTIONAL FRAMING TO CONFORM WITH PART 9 O.B.C. LATEST EDITION. ROOF RAFTERS THAT CROSS OVER TRUSSES TO BE 2X4 SPF #2 @ 24" O.C. WITH A VERT. POST TO THE TRUSS UNDERNEATH AT EACH CROSS POINT. VERT. POSTS LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END POINTS & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

Mitek V. 8.2.0

CONVENTIONAL FRAMING OTHERS



Job Track: **45147**
Layout ID: **336335**
Plan Log: **113188**

Builder / Location:
GOLD PARK HOMES / VAUGHAN

Project: **PINE VALLEY**

Date: **9/29/2021**

Designer: **JBURROWS/C**

Model / Elevation:

BLOCK 15

/ UNIT 1-5

THESE DRAWINGS CONSTITUTE THE PROPERTY OF ALPA ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY ALPA ROOF TRUSSES INC AND WILL BE RETRACTED BY ALPA ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

EWP DESIGN INC.

(905) 832-2250

FAX (905) 832-0286

RESPONSIBILITIES AND SPECIFICATIONS

RESPONSIBILITIES

1. EWP DESIGN INC. is responsible for the design of trusses as individual components.
2. It is the responsibility of others to ascertain that the design loads utilized on each drawing meet or exceed the actual dead load imposed by the structure, the live load imposed by the intended use and the snow load imposed by local building code or authorities with jurisdictions.
3. All dimensions are to be verified by the owner, contractor, architect or other authorities with jurisdictions before truss fabrication.
4. EWP DESIGN INC. bears no responsibility for the erection of trusses. Persons erecting trusses are cautioned to seek professional advice regarding the temporary and permanent bracing for the system. Bracing shown on EWP DESIGN INC. drawing is specified for the truss as a component only and forms an integral part of the truss design.
5. It is the truss manufacturer's responsibility to ensure that trusses are manufactured in conformance with specifications of EWP DESIGN INC. as outlined below.

SPECIFICATIONS

1. Trusses designed by EWP DESIGN INC. conform to the relevant section of the Ontario Building Code of Canada (Part 9 or Part 4) or to the Canadian code for farm buildings, whichever applies to the building type, as indicated on the EWP DESIGN INC. drawings, and conform to the design procedures established by the Truss Plate Institute of Canada. Unit stresses used for truss designs are as per the edition of CSA-O86 shown on EWP DESIGN INC. drawings.
2. Lumber is to be the size, species and grade as specified on EWP DESIGN INC. drawings.
3. Moisture content of lumber shall not exceed 19% in service unless specified otherwise.
4. Metal connector plates shall be applied to both faces of truss at each joint and shall be positioned as specified.
5. Top chords of trusses are assumed to be continuously braced laterally by roof sheathing or by purlins at intervals not exceeding 12.5 times the thickness of top chord member.
6. Bottom chords shall be laterally braced at intervals not exceeding 3M (10') o.c., where rigid ceiling is not applied directly to the underside of chords.

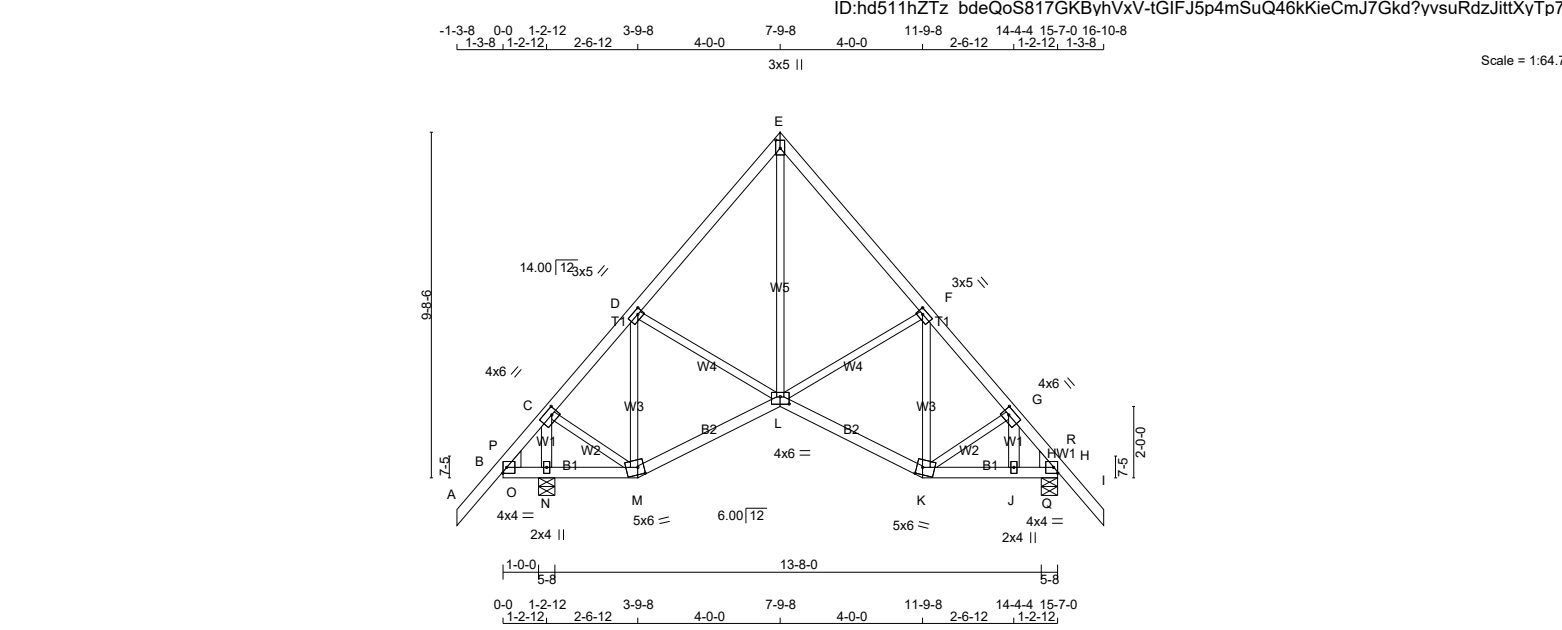
THESE DRAWINGS CONSTITUTE THE PROPERTY OF EWP DESIGN INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY THE ALPA LUMBER GROUP, AND WILL BE RETRACTED BY EWP DESIGN INC. IF UTILIZED FOR ANY OTHER PURPOSE.

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
336324	H81		1	TRUSS DESC. JT 45147	E21104078

Alpa Roof Truss, Maple

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed Oct 13 14:51:43 2021 Page 1

ID:hd511hZTz bdeQoS817GKByhVxV-tGIFJ5p4mSuQ46kKieCmJ7Gkd?yvsuRdzJittXyT7



TOTAL WEIGHT = 3 X 81 = 243 lb

[M][F]

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - E	2x4	DRY	No.2 SPF
E - I	2x4	DRY	No.2 SPF
B - M	2x4	DRY	No.2 SPF
M - L	2x4	DRY	No.2 SPF
L - K	2x4	DRY	No.2 SPF
K - H	2x4	DRY	No.2 SPF
ALL WEBS	2x3	DRY	No.2 SPF
EXCEPT			
J - G	2x4	DRY	No.2 SPF
N - C	2x4	DRY	No.2 SPF
DRY: SEASONED LUMBER.			

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMBH-I	MT20	4.0	4.0 Edge
C	TMWW-t	MT20	4.0	6.0 2.00 2.00
D	TMWW-t	MT20	3.0	5.0 1.50 1.75
E	TTW+p	MT20	3.0	5.0 2.75 1.50
F	TMWW-t	MT20	3.0	5.0 1.50 1.75
G	TMWW-t	MT20	4.0	6.0 2.00 2.00
H	TMBH-I	MT20	4.0	4.0 Edge
J	BMW+w	MT20	2.0	4.0
K	BBWW-m	MT20	5.0	6.0 2.50 2.00
L	BBWWW-p	MT20	4.0	6.0 2.50 3.00
M	BBWW-m	MT20	5.0	6.0 2.50 2.00
N	BMW1+w	MT20	2.0	4.0
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.				

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG		HEEL WEDGE	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	1-8	1-8	2x6 R	2x6 L
H	787	0	787	0	0	5-8	1-8				
N	935	0	935	0	0	5-8	1-8				

UNFACTORED REACTIONS		1ST LCASE		MAX./MIN. COMPONENT REACTIONS		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	206 / 0	0 / 0	245 / 0	0 / 0
H	559	353 / 0	0 / 0	0 / 0	0 / 0	206 / 0	0 / 0				
N	664	419 / 0	0 / 0	0 / 0	0 / 0	245 / 0	0 / 0				

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING		TOTAL LOAD CASES: (4)	
CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)
FR-TO	FROM	TO	FR-TO
A-B	0 / 39	-78.0 -78.0	10.00 L-E
B-P	0 / 145	-78.0 -78.0	10.00 L-F
P-C	0 / 215	-78.0 -78.0	10.00 K-F
C-D	-409 / 0	-78.0 -78.0	6.25 K-G
D-E	-500 / 0	-78.0 -78.0	6.25 J-G
E-F	-501 / 0	-78.0 -78.0	6.25 D-L
F-G	-596 / 0	-78.0 -78.0	6.25 M-D
G-R	-652 / 0	-78.0 -78.0	6.25 C-M
R-H	-960 / 0	-78.0 -78.0	6.25 N-C
H-I	0 / 39	-78.0 -78.0	10.00 O-P
B-O	-147 / 0	-18.5 -18.5	6.25 Q-R
O-N	-147 / 0	-18.5 -18.5	6.25
N-M	-147 / 0	-18.5 -18.5	6.25
M-L	0 / 313	-18.5 -18.5	10.00
L-K	0 / 462	-18.5 -18.5	10.00
K-J	0 / 387	-18.5 -18.5	10.00
J-Q	0 / 387	-18.5 -18.5	10.00
Q-H	0 / 387	-18.5 -18.5	10.00

DESIGN CRITERIA	
SPECIFIED LOADS:	
TOP CH. LL	= 21.0 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 34.4 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.48")

CALCULATED VERT. DEFL.(LL)= L/999 (0.01")

ALLOWABLE DEFL.(TL)= L/360 (0.48")

CALCULATED VERT. DEFL.(TL)= L/999 (0.03")

CANTILEVER DEFLECTION:

ALLOWABLE DEFL.(LL)= L/120 (0.19")

CALCULATED VERT. DEFL.(LL)= L/999 (0.00")

ALLOWABLE DEFL.(TL)= L/120 (0.19")

CALCULATED VERT. DEFL.(TL)= L/999 (0.00")

CSI: TC=0.15/1.00 (D-E:1), BC=0.14/1.00 (J-Q:1), WB=0.13/1.00 (D-M:1), SSI=0.17/1.00 (H-Q:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	
PLATE GRIP(DRY)	SHEAR SECTION
(PSI)	(PLI)
MAX MIN	MAX MIN
MT20	650 371 1747 788 1987 1873
PLATE PLACEMENT TOL. = 0.250 inches	
PLATE ROTATION TOL. = 5.0 Deg.	
JSI GRIP= 0.73 (M) (INPUT = 0.90)	
JSI METAL= 0.31 (B) (INPUT = 1.00)	

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

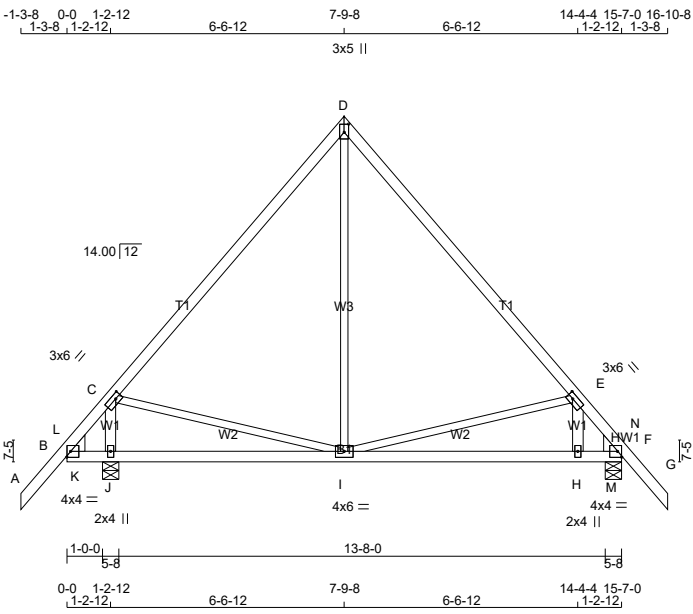


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
336324	H82		1	TRUSS DESC. JT 45147	E21104079

Alpa Roof Truss, Maple

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed Oct 13 14:51:45 2021 Page 1

ID:hd511hZTz bdeQoS817GKByhVxV-qfQ0knrKI488JQtjp3EEOYL TpbrKlxwQdB xQyTp6y



Scale = 1:64.7

TOTAL WEIGHT = 73 lb
[M][F]

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER		DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS 2x3 DRY No.2 SPF				
EXCEPT				
H - E	2x4	DRY	No.2	SPF
J - C	2x4	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMBH-I	MT20	4.0	4.0	Edge
C	TMWW-t	MT20	3.0	6.0	1.50 2.00
D	TTW+p	MT20	3.0	5.0	2.75 1.50
E	TMWW-t	MT20	3.0	6.0	1.50 2.00
F	TMBH1-I	MT20	4.0	4.0	Edge
H	BMW+w	MT20	2.0	4.0	
I	BMWWW-t	MT20	4.0	6.0	
J	BMW1+w	MT20	2.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG		HEEL WEDGE	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	1-8	2x6 R
F	787	0	787	0	0	5-8	1-8		
J	935	0	935	0	0	5-8	1-8	2x6 L	

UNFACTORED REACTIONS							
JT	1ST LCASE	MAX.	MIN.	COMPONENT	REACTIONS		
F	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	559	353 / 0	0 / 0	0 / 0	0 / 0	206 / 0	0 / 0
J	664	419 / 0	0 / 0	0 / 0	0 / 0	245 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.24 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)		
FR-TO		FROM	TO		FR-TO				
A-B	0 / 39	-78.0	-78.0	0.11 (1)	10.00	I-D	0 / 186	0.06 (4)	
B-L	-391 / 0	-78.0	-78.0	0.08 (1)	6.25	I-E	-375 / 0	0.32 (1)	
L-C	-4 / 177	-78.0	-78.0	0.52 (1)	10.00	H-E	-142 / 66	0.02 (4)	
C-D	-431 / 0	-78.0	-78.0	0.43 (1)	6.25	C-I	0 / 192	0.04 (1)	
D-E	-429 / 0	-78.0	-78.0	0.43 (1)	6.25	J-C	-981 / 0	0.10 (1)	
E-N	-709 / 0	-78.0	-78.0	0.36 (1)	6.25	K-L	0 / 645	0.00 (1)	
N-F	-1521 / 0	-78.0	-78.0	0.10 (1)	5.24	M-N	0 / 939	0.00 (1)	
F-G	0 / 39	-78.0	-78.0	0.11 (1)	10.00				
B-K	0 / 67	-18.5	-18.5	0.20 (1)	10.00				
K-J	0 / 67	-18.5	-18.5	0.20 (1)	10.00				
J-I	0 / 67	-18.5	-18.5	0.18 (4)	10.00				
I-H	0 / 617	-18.5	-18.5	0.23 (4)	10.00				
H-M	0 / 617	-18.5	-18.5	0.37 (1)	10.00				
M-F	0 / 617	-18.5	-18.5	0.37 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.48")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.48")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.52/1.00 (C-L:1) , BC=0.37/1.00 (F-M:1) ,
WB=0.32/1.00 (E-I:1) , SSI=0.57/1.00 (F-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES					
PLATE	GRIP(DRY)	SHEAR	SECTION		
	(PSI)	(PLI)	(PLI)		
	MAX	MIN	MAX	MIN	MAX
MT20	650	371	1747	788	1987

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.68 (E) (INPUT = 0.90)
JSI METAL= 0.33 (F) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

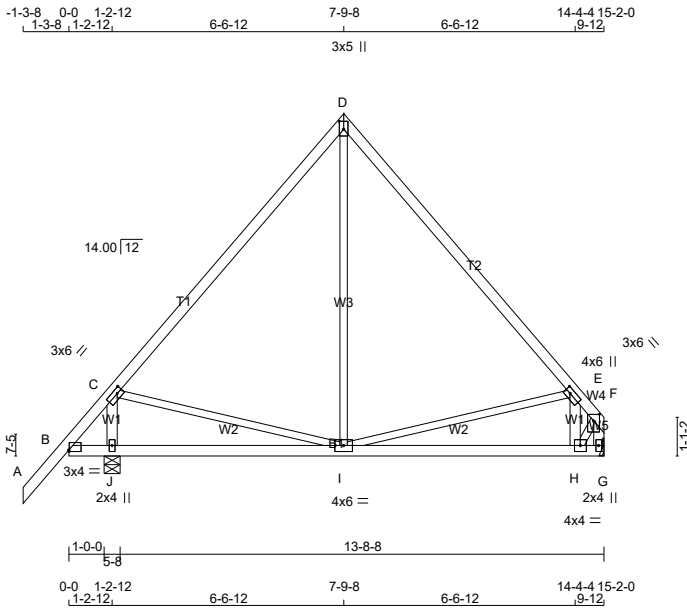


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
336324	H82S		1	TRUSS DESC. JT 45147	E21104080

Alpa Roof Truss, Maple

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed Oct 13 14:51:47 2021 Page 1

ID:hd511hZTz bdeQoS817GKByhVxV-m2Ym9TsbqhOsZj15xUHITzQLbclhogLDuxq4?lyTp6w



TOTAL WEIGHT = 2 X 70 = 139 lb

[M][F]

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - D	2x4	No.2	SPF	
D - F	2x4	DRY No.2	SPF	
G - F	2x4	DRY No.2	SPF	
B - G	2x4	DRY No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY No.2	SPF	
J - C	2x4	DRY No.2	SPF	
H - E	2x4	DRY No.2	SPF	
DRY: SEASONED LUMBER.				

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMB-I	MT20	3.0	4.0	0.50	0.25
C	TMWW-t	MT20	3.0	6.0	1.50	2.00
D	TTW+p	MT20	3.0	5.0	2.75	1.50
E	TMWW-t	MT20	3.0	6.0	1.50	2.00
F	TMVW+p	MT20	4.0	6.0	2.25	2.00
G	BMV1+p	MT20	2.0	4.0		
H	BMWW-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG	
	VERT	HORZ	DOWN	HORZ		IN-SX	IN-SX
JT	652	0	652	0	MECHANICAL		
G	652	0	652	0	5-8	1-8	
J	921	0	921	0			

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 1-8.

UNFACTORED REACTIONS

	MAX./MIN. COMPONENT REACTIONS						
	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	465	282 / 0	0 / 0	0 / 0	0 / 0	183 / 0	0 / 0
G	654	413 / 0	0 / 0	0 / 0	0 / 0	240 / 0	0 / 0
J							

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD			MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED		
		VERT.	LC1	MAX			UNBRAC	MAX	CSI (LC)
FR-TO		FROM	TO	LENGTH	FR-TO				
A-B	0 / 39	-78.0	-78.0	0.11 (1)	10.00	J-C	-801 / 0	0.08 (1)	
B-C	-25 / 54	-78.0	-78.0	0.48 (1)	6.25	C-I	0 / 175	0.04 (1)	
C-D	-411 / 0	-78.0	-78.0	0.43 (1)	6.25	I-D	0 / 160	0.05 (4)	
D-E	-413 / 0	-78.0	-78.0	0.43 (1)	6.25	I-E	-307 / 0	0.26 (1)	
E-F	-489 / 0	-78.0	-78.0	0.38 (1)	6.25	H-E	-367 / 0	0.04 (1)	
G-F	-696 / 0	0.0	0.0	0.07 (1)	7.81	H-F	0 / 723	0.16 (1)	
B-J	0 / 72	-18.5	-18.5	0.14 (4)	10.00				
J-I	0 / 72	-18.5	-18.5	0.18 (4)	10.00				
I-H	0 / 539	-18.5	-18.5	0.22 (4)	10.00				
H-G	0 / 0	-18.5	-18.5	0.12 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 21.0	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 34.4	PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.46")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.46")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.48/1.00 (B-C:1) , BC=0.22/1.00 (H-I:4) , WB=0.26/1.00 (E-I:1) , SSI=0.24/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES					
PLATE	GRIP(DRY)	SHEAR	SECTION		
	(PSI)	(PLI)	(PLI)		
	MAX	MIN	MAX	MIN	MAX
MT20	650	371	1747	788	1987

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (H) (INPUT = 0.90)
JSI METAL= 0.20 (H) (INPUT = 1.00)

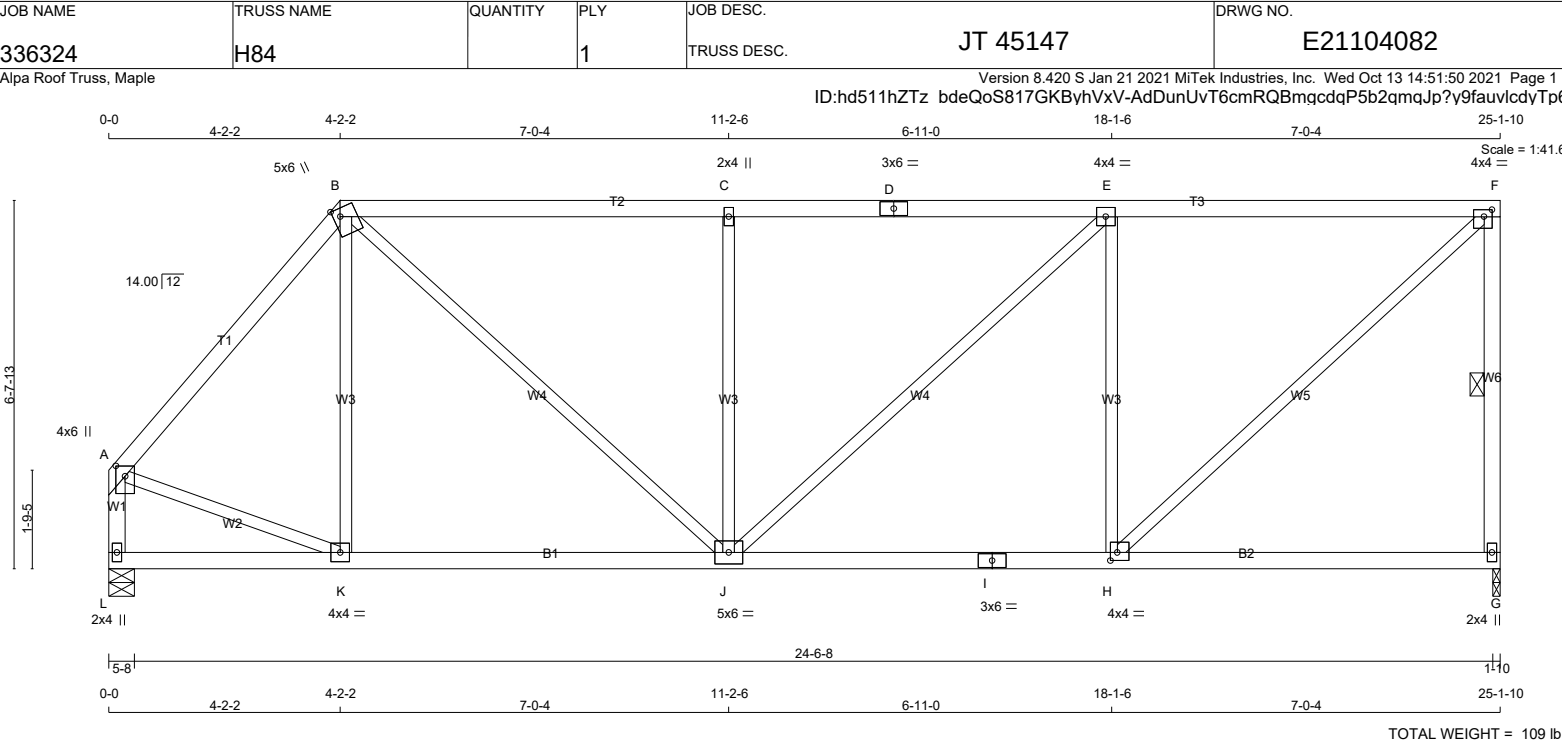
LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



The drawing illustrates a bridge truss system with the following components:

- Members:**
 - Top chord: T1, T2, T3
 - Bottom chord: W1, W2, W3, W4, W5, W6, W7, W8
 - Diagonal members: W1, W2, W3, W4, W5, W6, W7, W8
 - Vertical members: W1, W2, W3, W4, W5, W6, W7, W8
- Joints:** A, B, C, D, E, F, G, H, I, J, K, L, M, N, O, P, Q, R, S, T, U, V, W, X, Y, Z
- Supports:**
 - Pin support at joint A.
 - Roller support at joint K.
 - Pin support at joint H.
- Dimensions:**
 - Horizontal dimensions: 0-0, 3-0-7, 3-0-7, 5-7-9, 8-8-0, 5-7-9, 14-3-9, 5-3-3, 19-6-12, 5-5-12, 25-0-8, 25-3-8.
 - Vertical dimensions: 14-00'12", 5-3-13, 5-8'.
- Scale:** 1:41.9

CONTINUED ON PAGE 2



LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - A	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.25	2.00
B	TTWW+m	MT20	5.0	6.0	1.75	1.50
C	TMW+w	MT20	2.0	4.0		
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMVW-t	MT20	4.0	4.0	1.50	1.75
G	BMV1+p	MT20	2.0	4.0		
H	BMWW-t	MT20	4.0	4.0	1.75	1.50
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	5.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMV1+p	MT20	2.0	4.0		

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX		REQRD BRG IN-SX	
JT		VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
G	1212	0		1212	0	0	1-10	1-8	
L	1212	0		1212	0	0	5-8	1-8	

		1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT		COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
G	864	528 / 0		0 / 0	0 / 0	0 / 0	337 / 0	0 / 0	
L	864	528 / 0		0 / 0	0 / 0	0 / 0	337 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) (G, L)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

		CHORDS		FACTORED		WEBS		MAX. FACTORED	
MEMB.		FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	CS1 (LC)	MEMB.		FORCE (LBS)	MAX CS1 (LC)
FR-TO			FROM TO			FR-TO			
A- B	-1052 / 0		-78.0 -78.0	0.27 (1)	5.80	K- B	-105 / 42		0.08 (1)
B- C	-1251 / 0		-78.0 -78.0	0.61 (1)	4.92	B- J	0 / 770		0.17 (1)
C- D	-1252 / 0		-78.0 -78.0	0.61 (1)	4.91	J- C	-589 / 0		0.43 (1)
D- E	-1252 / 0		-78.0 -78.0	0.61 (1)	4.91	J- E	0 / 291		0.07 (1)
E- F	-1037 / 0		-78.0 -78.0	0.59 (1)	5.30	H- E	-794 / 0		0.58 (1)
G- F	-1160 / 0		0.0 0.0	0.24 (1)	5.91	H- F	0 / 1399		0.31 (1)
L- A	-1188 / 0		0.0 0.0	0.13 (1)	7.32	A- K	0 / 717		0.16 (1)
L- K	0 / 0		-18.5 -18.5	0.15 (4)	10.00				
K- J	0 / 679		-18.5 -18.5	0.23 (4)	10.00				
J- I	0 / 1037		-18.5 -18.5	0.32 (4)	10.00				
I- H	0 / 1037		-18.5 -18.5	0.32 (4)	10.00				
H- G	0 / 0		-18.5 -18.5	0.22 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.84")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.84")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.10")

CSI: TC=0.61/1.00 (C-E:1) , BC=0.32/1.00 (H-J:4) , WB=0.58/1.00 (E-H:1) , SSI=0.26/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

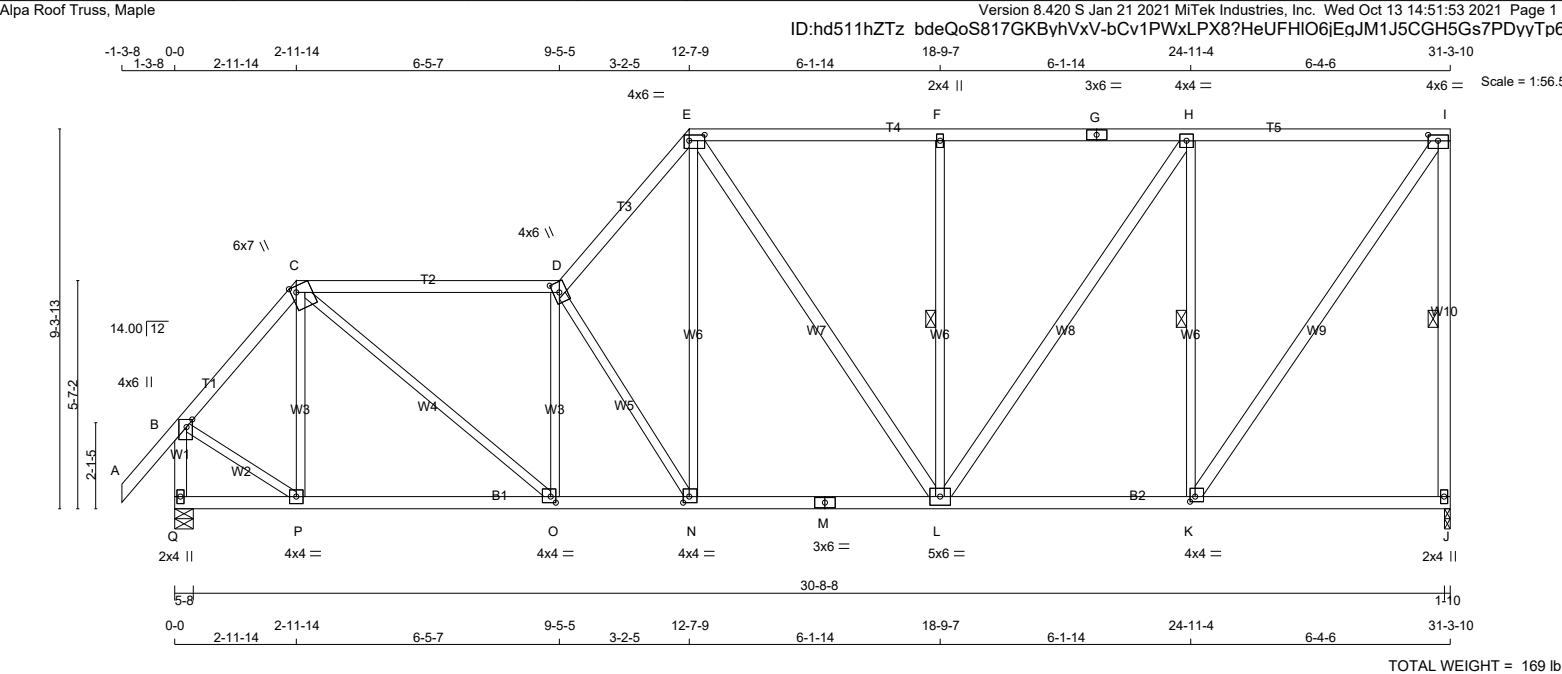
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (F) (INPUT = 0.90)
JSI METAL= 0.39 (A) (INPUT = 1.00)

JSI GRIP= 0.88 (C) (INPUT = 0.90)
JSI METAL= 0.48 (M) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
336324	H86		1	TRUSS DESC. JT 45147	E21104084



LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA			
N. L. G. A. RULES				BEARINGS				SPECIFIED LOADS:			
CHORDS	SIZE	LUMBER	DESCR.	FACTORED	MAXIMUM FACTORED	INPUT	REQRD	TOP CH.	LL = 21.0	PSF	
A - C	2x4	DRY	No.2	GROSS REACTION	GROSS REACTION	BRG	BRG	DL = 6.0	PSF		
C - D	2x4	DRY	No.2	JT VERT	HORZ	DOWN	HORZ	BOT CH.	LL = 0.0	PSF	
D - E	2x4	DRY	No.2	J 1510	0	1510	0	DL = 7.4	PSF		
E - G	2x4	DRY	No.2	Q 1619	0	1619	0	TOTAL LOAD = 34.4	PSF		
G - I	2x4	DRY	No.2	UNFACTORED REACTIONS				SPACING = 24.0 IN. C/C			
J - I	2x4	DRY	No.2	1ST LCASE	MAX./MIN. COMPONENT REACTIONS				LOADING IN ALL FLAT SECTIONS BASED ON A		
Q - B	2x4	DRY	No.2	JT COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
Q - M	2x4	DRY	No.2	J 1076	657 / 0	0 / 0	0 / 0	0 / 0	419 / 0	0 / 0	
M - J	2x4	DRY	No.2	Q 1152	716 / 0	0 / 0	0 / 0	0 / 0	436 / 0	0 / 0	
ALL WEBS	2x3	DRY	No.2	BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, Q				THIS TRUSS IS DESIGNED FOR RESIDENTIAL			
EXCEPT				BRACING				OR SMALL BUILDING REQUIREMENTS OF PART			
E - L	2x4	DRY	No.2	TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.00 FT.				9, NBCC 2015			
L - H	2x4	DRY	No.2	MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.				THIS DESIGN COMPLIES WITH:			
K - I	2x4	DRY	No.2	ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.				- PART 9 OF BCBC 2018 , ABC 2019			
DRY: SEASONED LUMBER.				1 LATERAL BRACE(S) AT 1/2 LENGTH OF I-J, F-L, H-K.				- PART 9 OF OBC 2012 (2019 AMENDMENT)			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.25	1.75
C	TTWW+m	MT20	6.0	7.0	Edge	1.50
D	TTWW+m	MT20	4.0	6.0	3.00	1.75
E	TTWW-l	MT20	4.0	6.0	1.75	4.50
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0		
I	TMVW-t	MT20	4.0	6.0	1.75	3.00
J	BMV1+p	MT20	2.0	4.0		
K	BMWW-t	MT20	4.0	4.0	1.50	1.50
L	BMWWW-t	MT20	5.0	6.0		
M	BS-t	MT20	3.0	6.0		
N	BMWW-t	MT20	4.0	4.0	1.75	1.75
O	BMWW-t	MT20	4.0	4.0	1.75	1.50
P	BMWW-t	MT20	4.0	4.0		
Q	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



CHORDS				WEBS			
MEMB.	FORCE (LBS)	FACTORED (PLF)	MAX. UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX. UNBRAC LENGTH	MAX. FACTORED (LC)
FR-TO	FROM	TO		FR-TO	FROM	TO	
A-B	0 / 42	-78.0	-78.0 0.11 (1)	10.00	P-C	-334 / 0	0.16 (1)
B-C	-1229 / 0	-78.0	-78.0 0.14 (1)	5.63	C-O	0 / 1416	0.32 (1)
C-D	-1878 / 0	-78.0	-78.0 0.71 (1)	4.00	O-D	-823 / 0	0.39 (1)
D-E	-1931 / 0	-78.0	-78.0 0.20 (1)	4.68	D-N	-1202 / 0	0.81 (1)
E-F	-1294 / 0	-78.0	-78.0 0.46 (1)	5.10	N-E	0 / 1104	0.25 (1)
F-G	-1294 / 0	-78.0	-78.0 0.49 (1)	5.04	E-L	0 / 43	0.01 (1)
G-H	-1294 / 0	-78.0	-78.0 0.49 (1)	5.04	L-F	-514 / 0	0.29 (1)
H-I	-890 / 0	-78.0	-78.0 0.46 (1)	5.83	L-H	0 / 717	0.12 (1)
J-I	-1462 / 0	0.0	0.0 0.60 (1)	5.41	K-H	-1132 / 0	0.65 (1)
Q-B	-1611 / 0	0.0	0.0 0.19 (1)	6.52	K-I	0 / 1544	0.25 (1)
					B-P	0 / 912	0.21 (1)
Q-P	0 / 0	-18.5	-18.5 0.13 (4)	10.00			
P-O	0 / 788	-18.5	-18.5 0.21 (1)	10.00			
O-N	0 / 1894	-18.5	-18.5 0.34 (1)	10.00			
N-M	0 / 1270	-18.5	-18.5 0.28 (1)	10.00			
M-L	0 / 1270	-18.5	-18.5 0.28 (1)	10.00			
L-K	0 / 890	-18.5	-18.5 0.26 (4)	10.00			
K-J	0 / 0	-18.5	-18.5 0.18 (4)	10.00			

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.04")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (1.04")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.71/1.00 (C-D:1) , BC=0.34/1.00 (N-O:1) , WB=0.81/1.00 (D-N:1) , SSI=0.23/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

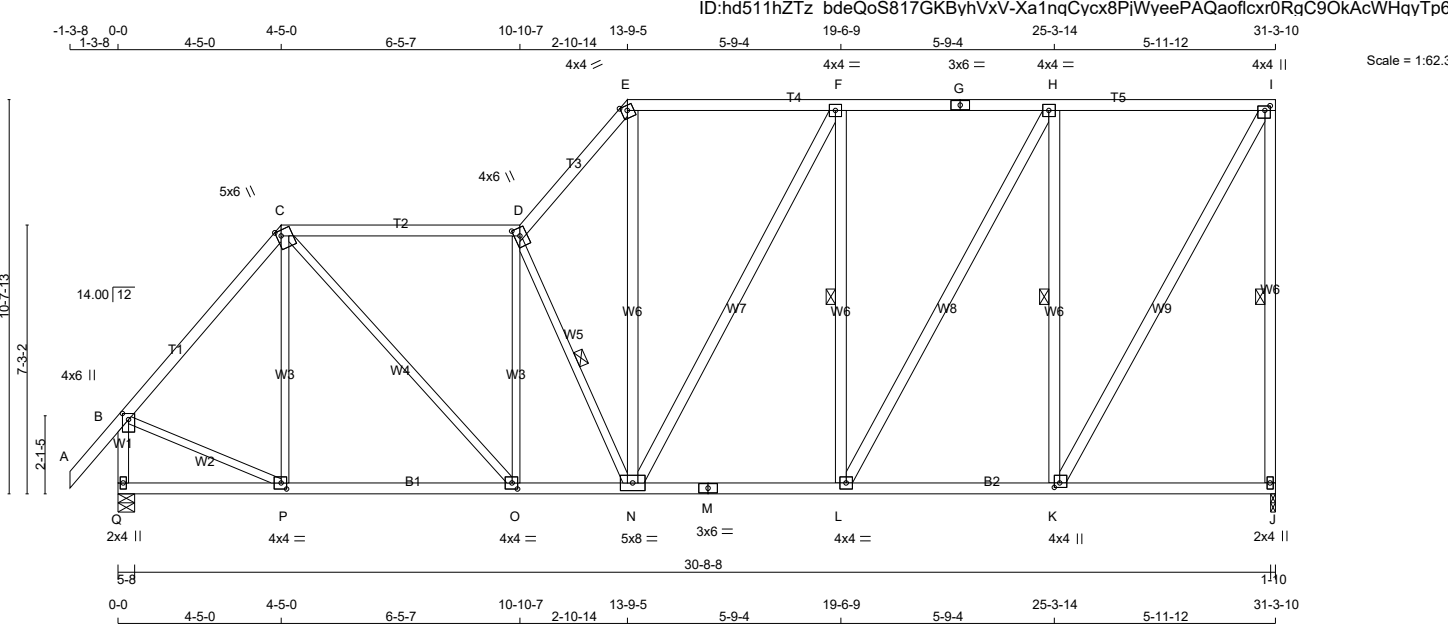
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (N) (INPUT = 0.90)
JSI METAL= 0.59 (E) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
336324	H87		1	TRUSS DESC. JT 45147	E21104085

Alpa Roof Truss, Maple

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed Oct 13 14:51:55 2021 Page 1



TOTAL WEIGHT = 194 lb [M]

LUMBER				N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF			
C - D	2x4	DRY	No.2	SPF			
D - E	2x4	DRY	No.2	SPF			
E - G	2x4	DRY	No.2	SPF			
G - I	2x4	DRY	No.2	SPF			
J - I	2x4	DRY	No.2	SPF			
Q - B	2x4	DRY	No.2	SPF			
Q - M	2x4	DRY	No.2	SPF			
M - J	2x4	DRY	No.2	SPF			
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF			
P - C	2x3	DRY	No.2	SPF			
C - O	2x3	DRY	No.2	SPF			
O - D	2x3	DRY	No.2	SPF			
D - N	2x3	DRY	No.2	SPF			
B - P	2x3	DRY	No.2	SPF			

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	4.0	6.0	2.00 2.00
C	TTWW+m	MT20	5.0	6.0	1.75 1.50
D	TTWW+m	MT20	4.0	6.0	2.50 1.75
E	TTW-m	MT20	4.0	4.0	Edge
F	TMWW-t	MT20	4.0	4.0	
G	TS-t	MT20	3.0	6.0	
H	TMWW-t	MT20	4.0	4.0	
I	TMVW+p	MT20	4.0	4.0	1.50 1.75
J	BMV1+p	MT20	2.0	4.0	
K	BMWW+t	MT20	4.0	4.0	1.50 1.75
L	BMWW-t	MT20	4.0	4.0	
M	BS-t	MT20	3.0	6.0	
N	BMWWW-t	MT20	5.0	8.0	
O	BMWW-t	MT20	4.0	4.0	2.00 1.75
P	BMWW-t	MT20	4.0	4.0	2.00 1.75
Q	BMV1+p	MT20	2.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD IN-SX	
JT	VERT	HORZ		DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
J	1510	0		1510	0	0	1-10	1-10	
Q	1619	0		1619	0	0	5-8	1-12	

UNFACTORED REACTIONS		1ST LCASE		MAX./MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND
J	1076	657 / 0	0 / 0	0 / 0	0 / 0
Q	1152	716 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, Q

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.39 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF I-J, D-N, F-L, H-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 MAX. (LC)	MEMB.	FORCE (LBS)	MAX. FACTORED UNBRAC LENGTH	MAX. FACTORED CSI (LC)
FR-TO		FROM TO		FR-TO			
A-B	0 / 42	-78.0 -78.0	0.11 (1)	10.00	P-C	-203 / 3	0.19 (1)
B-C	-1296 / 0	-78.0 -78.0	0.33 (1)	5.31	C-O	0 / 1039	0.23 (1)
C-D	-1538 / 0	-78.0 -78.0	0.67 (1)	4.39	O-D	-686 / 0	0.64 (1)
D-E	-1716 / 0	-78.0 -78.0	0.16 (1)	4.94	D-N	-1072 / 0	0.41 (1)
E-F	-1131 / 0	-78.0 -78.0	0.38 (1)	5.47	N-E	0 / 1000	0.16 (1)
F-G	-1098 / 0	-78.0 -78.0	0.41 (1)	5.47	N-F	0 / 69	0.01 (1)
G-H	-1098 / 0	-78.0 -78.0	0.41 (1)	5.47	L-F	-542 / 0	0.31 (1)
H-I	-737 / 0	-78.0 -78.0	0.40 (1)	6.25	L-H	0 / 741	0.12 (1)
J-I	-1465 / 0	0.0 0.0	0.84 (1)	5.41	K-H	-1155 / 0	0.66 (1)
Q-B	-1591 / 0	0.0 0.0	0.19 (1)	6.55	K-I	0 / 1475	0.24 (1)
Q-P	0 / 0	-18.5 -18.5	0.14 (4)	10.00	B-P	0 / 900	0.20 (1)
P-O	0 / 836	-18.5 -18.5	0.22 (4)	10.00			
O-N	0 / 1549	-18.5 -18.5	0.29 (1)	10.00			
N-M	0 / 1098	-18.5 -18.5	0.24 (1)	10.00			
M-L	0 / 1098	-18.5 -18.5	0.24 (1)	10.00			
L-K	0 / 737	-18.5 -18.5	0.23 (4)	10.00			
K-J	0 / 0	-18.5 -18.5	0.15 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.04")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (1.04")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.15")

CSI: TC=0.84/1.00 (I-J:1) , BC=0.29/1.00 (N-O:1) , WB=0.66/1.00 (H-K:1) , SSI=0.22/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (O) (INPUT = 0.90)
JSI METAL= 0.52 (D) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

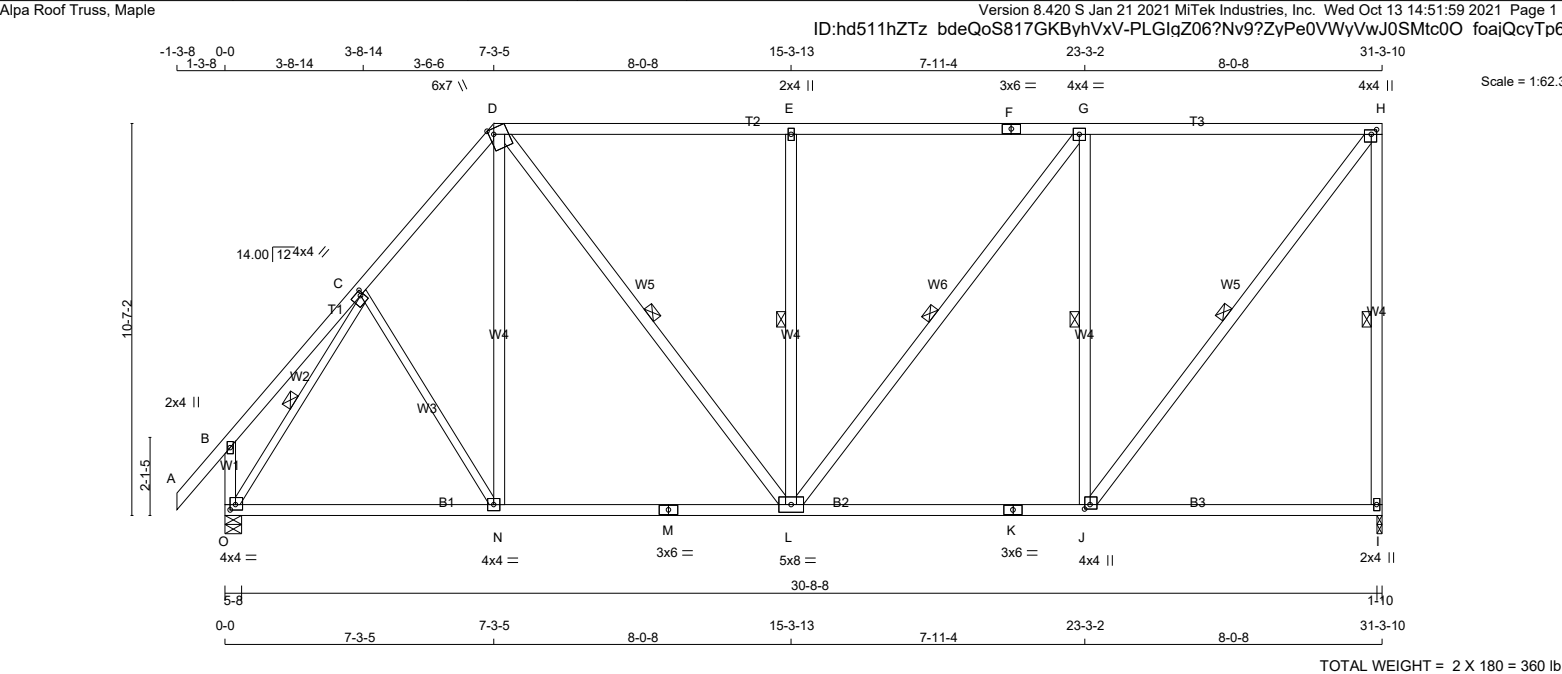


Scale = 1:72.



JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.52 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
336324	H89		1	TRUSS DESC. JT 45147	E21104087



LUMBER			DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER			DESIGN CRITERIA		
N. L. G. A. RULES			BEARINGS			SPECIFIED LOADS:		
CHORDS	SIZE	LUMBER	DESCR.	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG	TOP CH. LL = 21.0 PSF
A - D	2x4	DRY	No.2	VERT	HORZ	DOWN	HORZ	DL = 6.0 PSF
D - F	2x4	DRY	No.2	SPF	SPF	UPLIFT	IN-SX	BOT CH. LL = 0.0 PSF
F - H	2x4	DRY	No.2	SPF	SPF	IN-SX	IN-SX	DL = 7.4 PSF
I - H	2x4	DRY	No.2	SPF	SPF	1-10	1-10	TOTAL LOAD = 34.4 PSF
O - B	2x4	DRY	No.2	SPF	SPF	5-8	1-12	
O - M	2x4	DRY	No.2	SPF	SPF			
M - K	2x4	DRY	No.2	SPF	SPF			
K - I	2x4	DRY	No.2	SPF	SPF			
ALL WEBS	2x4	DRY	No.2	SPF	SPF			
EXCEPT								
C - N	2x3	DRY	No.2	SPF	SPF			
O - C	2x3	DRY	No.2	SPF	SPF			

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWW-t	MT20	4.0	4.0	1.50	1.00
D	TTWW+m	MT20	6.0	7.0	Edge	1.50
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TMVW+p	MT20	4.0	4.0	1.50	1.75
I	BMV1+p	MT20	2.0	4.0		
J	BMWW+t	MT20	4.0	4.0	1.50	1.75
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	5.0	8.0		
M	BS-t	MT20	3.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMVW1-t	MT20	4.0	4.0	1.75	1.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF H-I, D-L, E-L, G-L, G-J, H-J, C-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 42	-78.0 -78.0	0.11 (1)	C-N	0 / 48	0.02 (4)	
B-C	0 / 24	-78.0 -78.0	0.16 (1)	N-D	0 / 139	0.03 (4)	
C-D	-1308 / 0	-78.0 -78.0	0.20 (1)	D-L	0 / 597	0.10 (1)	
D-E	-1201 / 0	-78.0 -78.0	0.83 (1)	L-E	-678 / 0	0.38 (1)	
E-F	-1202 / 0	-78.0 -78.0	0.83 (1)	L-G	0 / 436	0.07 (1)	
F-G	-1202 / 0	-78.0 -78.0	0.83 (1)	J-G	-1030 / 0	0.58 (1)	
G-H	-936 / 0	-78.0 -78.0	0.79 (1)	J-H	0 / 1521	0.24 (1)	
H-I	-1451 / 0	0.0 0.0	0.82 (1)	O-C	-1576 / 0	0.51 (1)	
O-B	-217 / 0	0.0 0.0	0.03 (1)				
O-N	0 / 816	-18.5 -18.5	0.34 (4)				
N-M	0 / 833	-18.5 -18.5	0.35 (4)				
M-L	0 / 833	-18.5 -18.5	0.35 (4)				
L-K	0 / 936	-18.5 -18.5	0.38 (4)				
K-J	0 / 936	-18.5 -18.5	0.38 (4)				
J-I	0 / 0	-18.5 -18.5	0.29 (4)				

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



ALLOWABLE DEFL.(LL)= L/360 (1.04")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (1.04")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.13")

CSI: TC=0.83/1.00 (D-E:1), BC=0.38/1.00 (J-L:4),
WB=0.58/1.00 (G-J:1), SSI=0.29/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

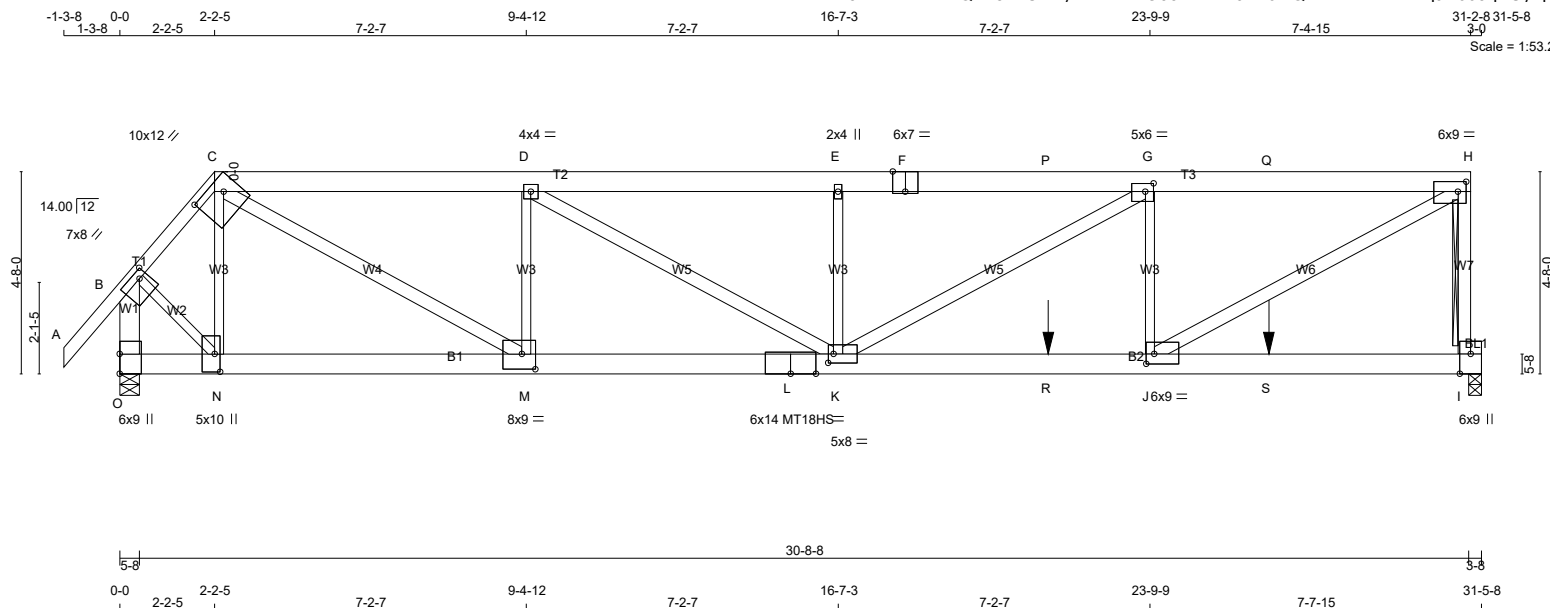
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.53 (C) (INPUT = 1.00)



TOTAL WEIGHT = 173 lb

LUMBER		N. L. G. A. RULES		LUMBER		DESCR	
CHORDS		SIZE					
A - C	2x4	DRY	No.2				SPF
C - F	2x6	DRY	1650F 1.5E				SPF
F - H	2x6	DRY	1650F 1.5E				SPF
I - H	2x4	DRY	No.2				SPF
O - B	2x6	DRY	No.2				SPF
O - L	2x6	DRY	2100F 1.8E				SPF
L - I	2x6	DRY	2100F 1.8E				SPF

BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF

ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - M	2x4	DRY	No.2	SPF
D - K	2x4	DRY	1650F 1.5E	SPF
K - G	2x4	DRY	1650F 1.5E	SPF
J - H	2x4	DRY	1650F 1.5E	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
PLATES	JT TYPE	PLATES	W	LEN	Y X
B	TMVW-t	MT20	7.0	8.0	2.00 2.25
C	TTWW-h	MT20	10.0	12.0	3.75 8.00
D	TMVW-t	MT20	4.0	4.0	
E	TMVW+w	MT20	2.0	4.0	
F	TS-t	MT20	6.0	7.0	Edge 3.50
G	TMVW-t	MT20	5.0	6.0	2.25 2.25
H	TMVW-t	MT20	6.0	9.0	2.75 2.25
I	BMVK1+t	MT20	6.0	9.0	Edge 3.00
J	BMVW-t	MT20	6.0	9.0	2.75 2.25
K	BMVWWW-t	MT20	5.0	8.0	2.50 1.50
L	BS-t	MT18HS	6.0	14.0	
M	BMVW-t	MT20	8.0	9.0	4.25 3.75
N	BMVW+t	MT20	5.0	10.0	5.00 1.50
O	BMV1+t	MT20	6.0	9.0	5.50

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

LATERAL BRACE(S) SHOWN SHALL BE
2X4 SPF#2



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
I	3675	0	3675	0	0	3-8	3-8
O	4844	0	4844	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	2620	1600 / 0	0 / 0	0 / 0	0 / 0	1020 / 0	0 / 0
O	3452	2119 / 0	0 / 0	0 / 0	0 / 0	1333 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O
BEARING SIZE FACTOR = 1.15 AT JNT(S) I, O (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.23 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT H-I

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

C H O R D S					W E B S				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. CSI (LC)	UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM	TO		LENGTH	FR-TO			
A-B	0 / 42	-78.0	-78.0	0.12	(1)	10.00	N-C	-260 / 0 0.08 (1)	
B-C	-3851 / 0	-78.0	-78.0	0.27	(1)	3.36	C-M	0 / 5660 1.00 (1)	
C-D	-7387 / 0	-78.0	-78.0	0.41	(1)	3.49	M-D	-914 / 0 0.27 (1)	
D-E	-7922 / 0	-78.0	-78.0	0.39	(1)	3.41	D-K	0 / 619 0.08 (1)	
E-F	-7922 / 0	-137.2	-137.2	0.57	(1)	3.23	K-E	-713 / 0 0.21 (1)	
F-P	-7922 / 0	-137.2	-137.2	0.57	(1)	3.23	K-G	0 / 2988 0.38 (1)	
P-G	-7922 / 0	-78.0	-78.0	0.57	(1)	3.23	J-H	-2372 / 0 0.70 (1)	
G-Q	-5342 / 0	-78.0	-78.0	0.43	(1)	3.95	J-G	0 / 6282 0.80 (1)	
Q-H	-5342 / 0	-130.8	-130.8	0.43	(1)	3.95	B-N	0 / 3092 0.77 (1)	
I-H	-3483 / 0	0.0	0.0	0.68	(1)	6.00			
O-B	-4925 / 0	0.0	0.0	0.40	(1)	4.81			
O-N	0 / 0	-256.7	-256.7	0.35	(1)	10.00			
N-M	0 / 2502	-256.7	-256.7	0.57	(1)	10.00			
M-L	0 / 7387	-256.7	-256.7	0.83	(1)	10.00			
L-K	0 / 7387	-256.7	-256.7	0.83	(1)	10.00			
K-R	0 / 5342	-33.5	-33.5	0.50	(1)	10.00			
R-J	0 / 5342	-18.5	-18.5	0.50	(1)	10.00			
J-S	-121 / 0	-18.5	-18.5	0.22	(1)	6.25			
S-I	-121 / 0	-31.0	-31.0	0.22	(1)	6.25			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
R	21-5-7	-393	-393	---	FRONT	VERT	TOTAL	---	C1
S	26-6-9	-393	-393	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED
BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP	CH.	LL =	21.0	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL LOAD			= 34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 2.00/12 MINIMUM

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 11-10-8
END DISTANCE = 16-6-0
END SPAN CARRIED = 11-10-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADD'T'L LOADS BASED ON 55 % OF GSL.

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 2-2-5
RIGHT SETBACK = 0-0
END SETBACK = 4-8-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADD'T'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 4-10-15 OF SPAN
MEASURED FROM THE RIGHT.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL
LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIQ 2014

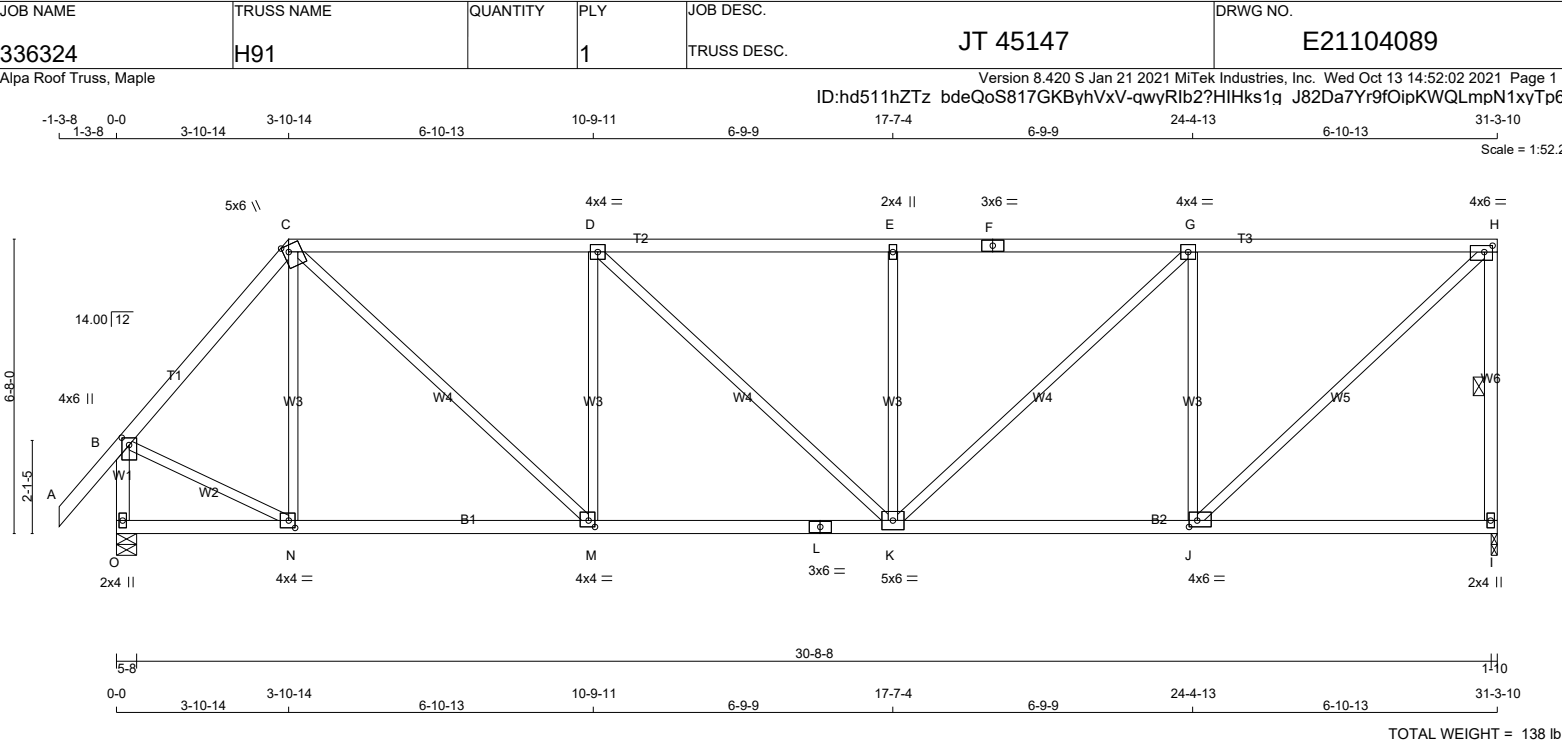
(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

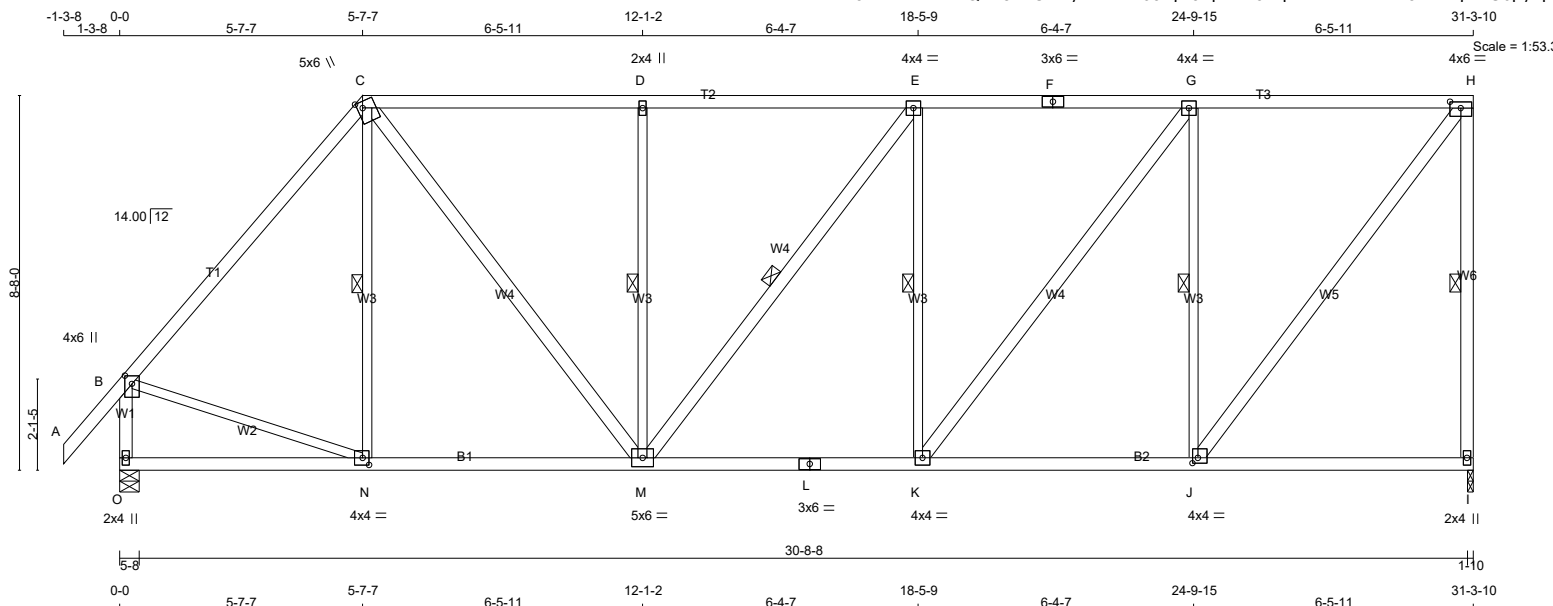
ALLOWABLE DEFL.(LL)= L/360 (1.04")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.33")
ALLOWABLE DEFL.(TL)= L/360 (1.04")
CALCULATED VERT. DEFL.(TL) = L/ 581 (0.64")

CSI: TC=0.68/1.00 (H-I:1) , BC=0.83/1.00 (K-M:1) ,
WB=1.00/1.00 (C-M:1) , SSI=0.66/1.00 (K-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00





TOTAL WEIGHT = 166 lb

LUMBER

N. L. G. A. RULES				
CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - M	2x4	DRY	No.2	SPF
M - E	2x4	DRY	No.2	SPF
K - G	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.25	2.00
C	TTWW+m	MT20	5.0	6.0	1.75	1.50
D	TMVW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMVW-t	MT20	4.0	4.0		
H	TMVW-t	MT20	4.0	6.0	1.75	3.00
I	BMV1+p	MT20	2.0	4.0		
J	BMVW-t	MT20	4.0	4.0	1.50	1.50
K	BMVW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMVWW-t	MT20	5.0	6.0		
N	BMVW-t	MT20	4.0	4.0	2.00	1.75
O	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
I	1510	0	1510	0	0	1-10	1-10
O	1619	0	1619	0	0	5-8	1-12

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1076	657 / 0	0 / 0	0 / 0	0 / 0	419 / 0	0 / 0
O	1152	716 / 0	0 / 0	0 / 0	0 / 0	436 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.79 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF H-I, C-N, D-M, E-M, E-K, G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

C H O R D S				W E B S				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM	TO	LENGTH	FR-TO			
A-B	0 / 42	-78.0	-78.0	0.11 (1)	10.00	N-C	-131 / 38	0.06 (1)
B-C	-1304 / 0	-78.0	-78.0	0.58 (1)	5.02	C-M	0 / 881	0.14 (1)
C-D	-1384 / 0	-78.0	-78.0	0.56 (1)	4.81	M-D	-563 / 0	0.27 (1)
D-E	-1385 / 0	-78.0	-78.0	0.55 (1)	4.81	M-E	-22 / 0	0.01 (1)
E-F	-1398 / 0	-78.0	-78.0	0.56 (1)	4.79	K-E	-442 / 0	0.21 (1)
F-G	-1398 / 0	-78.0	-78.0	0.56 (1)	4.79	K-G	0 / 698	0.11 (1)
G-H	-985 / 0	-78.0	-78.0	0.52 (1)	5.54	J-G	-1127 / 0	0.54 (1)
I-H	-1462 / 0	0.0	0.0	0.50 (1)	5.41	J-H	0 / 1595	0.26 (1)
O-B	-1578 / 0	0.0	0.0	0.19 (1)	6.57	B-N	0 / 884	0.20 (1)
O-N	0 / 0	-18.5	-18.5	0.16 (4)	10.00			
N-M	0 / 844	-18.5	-18.5	0.24 (4)	10.00			
M-L	0 / 1398	-18.5	-18.5	0.30 (1)	10.00			
L-K	0 / 1398	-18.5	-18.5	0.30 (1)	10.00			
K-J	0 / 975	-18.5	-18.5	0.28 (4)	10.00			
J-I	0 / 0	-18.5	-18.5	0.19 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL	=	21.0	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.4	PSF

TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= $L/360$ (1.04")
 CALCULATED VERT. DEFL.(LL) = $L/999$ (0.06")
 ALLOWABLE DEFL.(TL)= $L/360$ (1.04")
 CALCULATED VERT. DEFL.(TL) = $L/999$ (0.13")

CSI: TC=0.58/1.00 (B-C:1), BC=0.30/1.00 (K-M:1),
WB=0.54/1.00 (G-J:1), SSI=0.24/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.52 (B) (INPUT = 1.00)

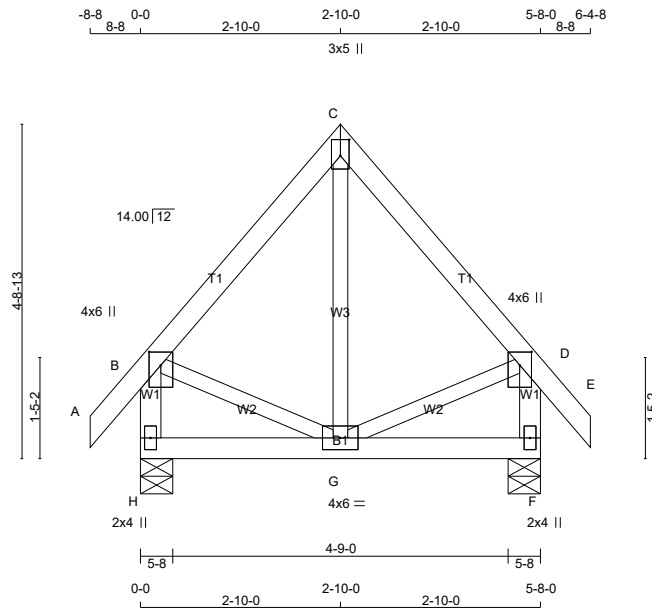
LATERAL BRACE(S) SHOWN SHALL BE
2X4 SPF#2



Alpa Roof Truss, Maple

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed Oct 13 14:52:07 2021 Page 1
ID:hd511hZTz bdeQoS817GKByhVxV-AuIKLI676qv0yoZx6heOHBFSkgA6Un49U2W8i8yTp6c

Scale = 1:32.6



TOTAL WEIGHT = 30 lb

N. L. G. A. RULES				
CHORDS	SIZE		LUMBER	DESCR
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
--------------------	-----	-----	------	-----

DRY: SEASONED LUMBER.

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.25	2.00
C	TTW+p	MT20	3.0	5.0	2.75	1.50
D	TMVW+p	MT20	4.0	6.0	2.25	2.00
F	BMV1+p	MT20	2.0	4.0		
G	BMVWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	2.0	4.0		

BEARINGS		FACTORED			MAXIMUM FACTORED			INPUT	REQD
	GROSS REACTION		GROSS REACTION			BRG		BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX		IN-SX	
H	337	0	337	0	0	5-8		1-8	
F	337	0	337	0	0	5-8		1-8	

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	239	153 / 0	0 / 0	0 / 0	0 / 0	86 / 0	0 / 0
F	239	153 / 0	0 / 0	0 / 0	0 / 0	86 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED

TOTAL LOAD CASES: (4)

CHORDS					WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		LENGTH	FR-TO			
A-B	0 / 24	-78.0	-78.0	0.04 (1)	10.00	G-C	-6 / 43 0.01 (4)	
B-C	-142 / 0	-78.0	-78.0	0.08 (1)	6.25	B-G	0 / 98 0.02 (1)	
C-D	-142 / 0	-78.0	-78.0	0.08 (1)	6.25	G-D	0 / 98 0.02 (1)	
D-E	0 / 24	-78.0	-78.0	0.04 (1)	10.00			
H-B	-316 / 0	0.0	0.0	0.03 (1)	7.81			
F-D	-316 / 0	0.0	0.0	0.03 (1)	7.81			
H-G	0 / 0	-18.5	-18.5	0.04 (4)	10.00			
G-F	0 / 0	-18.5	-18.5	0.04 (4)	10.00			

LATERAL BRACE(S) SHOWN SHALL BE
2X4 SPF#2



SPECIFIED LOADS:

TOP	CH.	LL	=	21.0	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.4	PSF
TOTAL LOAD				=	34.4 PSF

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.08/1.00 (C-D:1), BC=0.04/1.00 (F-G:4),
WB=0.02/1.00 (B-G:1), SSI=0.06/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

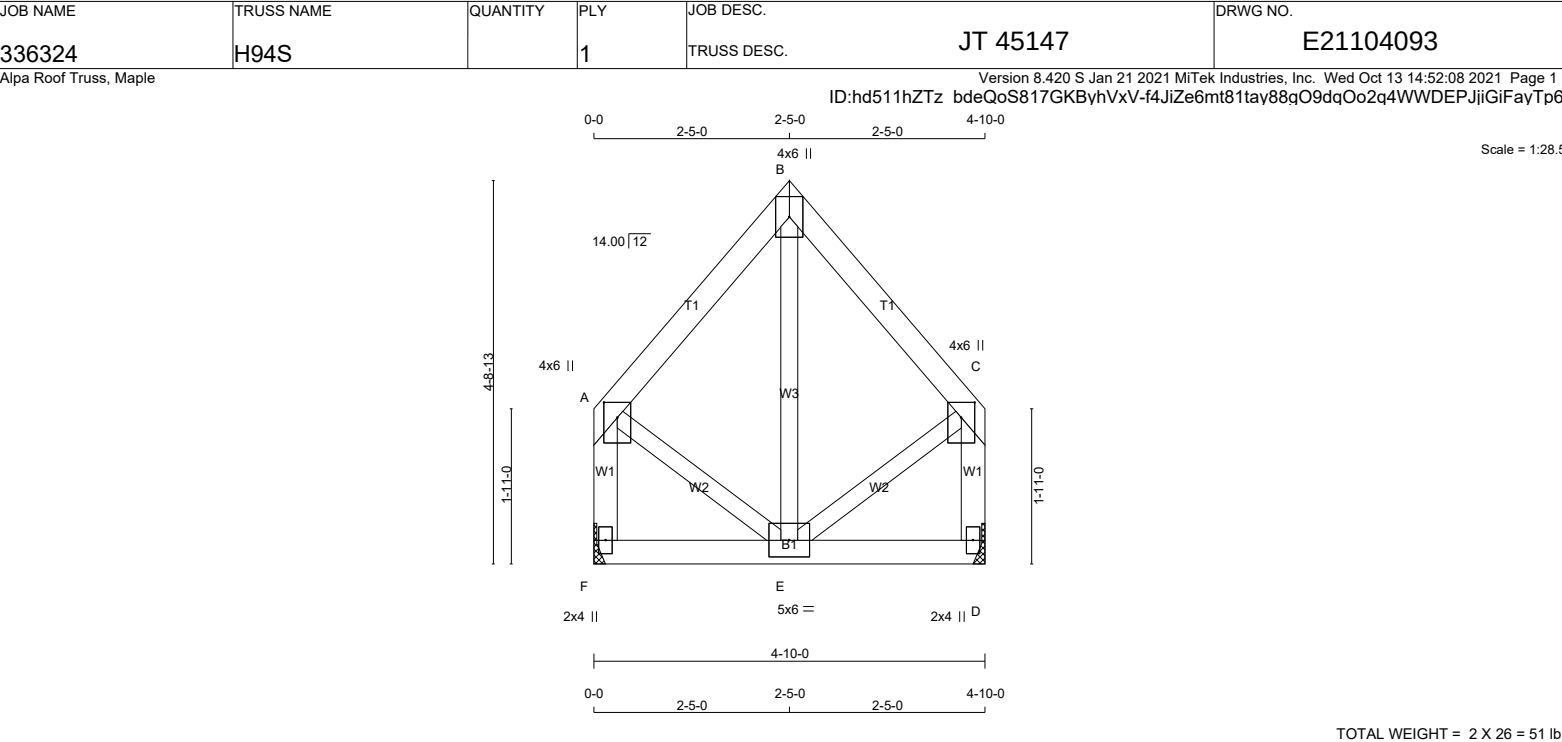
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES					
PLATE	GRIP(DRY)		SHEAR		SECTION
	(PSI)		(PLI)		(PLI)
	MAX	MIN	MAX	MIN	MAX MIN
MT20	650	371	1747	788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (D) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)

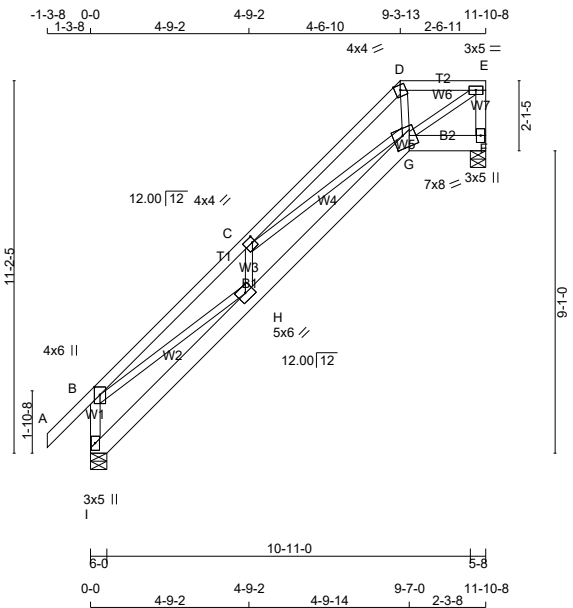


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
336324	H95		1	TRUSS DESC. JT 45147	E21104094

Alpa Roof Truss, Maple

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed Oct 13 14:52:10 2021 Page 1

ID:hd511hZTz bdeQoS817GKByhVxV-bTRS K80PIHbpFIWnpB5vptKZu92h4bcA0IoJTyTp6Z



Scale = 1:69.2

TOTAL WEIGHT = 66 lb
[M][F]

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
I - B	2x4	DRY No.2	SPF
A - D	2x4	DRY No.2	SPF
D - E	2x4	DRY No.2	SPF
F - E	2x4	DRY No.2	SPF
I - G	2x6	DRY No.2	SPF
G - F	2x6	DRY No.2	SPF
ALL WEBS 2x3 DRY No.2 SPF			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.00
D	TTW-m	MT20	4.0	4.0	Edge	
E	TMVW-t	MT20	3.0	5.0		
F	BMV1+p	MT20	3.0	5.0		
G	BBWWW-m	MT20	7.0	8.0	4.50	3.50
H	BMWW-t	MT20	5.0	6.0		
I	BMV1+p	MT20	3.0	5.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
JT						
I	682	0	682	0	6-0	1-8
F	573	0	573	0	5-8	1-8

UNFACTORED REACTIONS							
JT	COMBINED	MAX./MIN. COMPONENT REACTIONS			WIND	DEAD	SOIL
		1ST LCASE	SNOW	LIVE			
I	484	308 / 0	0 / 0	0 / 0	0 / 0	176 / 0	0 / 0
F	408	249 / 0	0 / 0	0 / 0	0 / 0	159 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.31 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD			MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED		
		FROM	TO	LC1 MAX CSI (LC)			UNBRAC LENGTH	FR-TO	
FR-TO									
I-B	-623 / 0	0.0	0.0	0.07 (1)	7.81	B-H	0 / 1174	0.26 (1)	
A-B	0 / 38	-78.0	-78.0	0.11 (1)	10.00	H-C	-257 / 0	0.04 (1)	
B-C	-1379 / 0	-78.0	-78.0	0.23 (1)	5.31	C-G	-440 / 0	0.29 (1)	
C-D	-839 / 0	-78.0	-78.0	0.21 (1)	6.25	D-G	0 / 329	0.07 (1)	
D-E	-624 / 0	-78.0	-78.0	0.07 (1)	6.25	G-E	0 / 775	0.17 (1)	
F-E	-552 / 0	0.0	0.0	0.06 (1)	7.81				
I-H	-9 / 2	-18.5	-18.5	0.07 (1)	10.00				
H-G	0 / 1342	-18.5	-18.5	0.22 (1)	10.00				
G-F	0 / 0	-18.5	-18.5	0.01 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.40")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.40")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.09")

CSI: TC=0.23/1.00 (B-C:1) , BC=0.22/1.00 (G-H:1) , WB=0.29/1.00 (C-G:1) , SSI=0.12/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (E) (INPUT = 0.90)
JSI METAL= 0.59 (B) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed Oct 13 14:52:12 2021 Page 1



TOTAL WEIGHT = 8 X 69 = 556 lb

PLATES (table is in inches)							
PLATES	JT	TYPE	PLATES	W	LEN	Y	X
B	CTMWW+p		MT20	4.0	6.0	2.75	2.00
C	CTMWW-t		MT20	4.0	4.0	2.00	1.00
D	TS-t		MT20	3.0	6.0		
E	ETMWW-t		MT20	4.0	4.0	2.00	1.00
F	TMV+p		MT20	2.0	4.0		
G	BMWV1-t		MT20	3.0	4.0	1.50	1.75
H	BWVV+p		MT20	7.0	8.0	3.25	2.75
I	BMWV-t		MT20	5.0	6.0		
J	BMV1+p		MT20	3.0	5.0		

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 1-8.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.29 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED MAX. CSI (LC)	
FR-TO		FROM TO			FR-TO			
J-B	-622 / 0	0.0	0.0	0.07 (1)	7.81	B-I	0 / 1181	0.27 (1)
A-B	0 / 38	-78.0	-78.0	0.11 (1)	10.00	I-C	-242 / 0	0.04 (1)
B-C	-1883 / 0	-78.0	-78.0	0.24 (1)	5.29	C-H	-416 / 0	0.29 (1)
C-D	-830 / 0	-78.0	-78.0	0.19 (1)	6.25	H-E	0 / 819	0.18 (1)
D-E	-850 / 0	-78.0	-78.0	0.19 (1)	6.25	E-G	-770 / 0	0.14 (1)
E-F	-35 / 0	-78.0	-78.0	0.11 (1)	6.25			
G-F	-36 / 0	0.0	0.0	0.01 (1)	7.81			
J-I	-9 / 2	-18.5	-18.5	0.07 (1)	10.00			
I-H	0 / 1342	-18.5	-18.5	0.22 (1)	10.00			
H-G	0 / 553	-18.5	-18.5	0.11 (1)	10.00			

SPECIFIED LOADS:

TOP	CH.	LL	=	21.0	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.4	PSF
TOTAL LOAD				=	34.4 PSF

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.40")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.40")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.24/1.00 (B-C:1) , BC=0.22/1.00 (H-I:1)
WB=0.29/1.00 (C-H:1) , SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES						
PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (E) (INPUT = 0.90)
JSI METAL= 0.59 (B) (INPUT = 1.00)

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed Oct 13 14:52:13 2021 Page 1



TOTAL WEIGHT = 6 X 47 = 279 lb

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER							
<u>BEARINGS</u>							
JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	346	0	346	0	0	MECHANICAL	
F	454	0	454	0	0	5-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

SPECIFIED LOADS:

TOP	CH.	LL	=	21.0	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.4	PSF
TOTAL LOAD				=	34.4 PSF

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.24")
CALCULATED VERT. DEFL.(TL) = L/ 723 (0.12")

CSI: TC=0.17/1.00 (B-C:1), BC=0.28/1.00 (E-F:4),
WB=0.19/1.00 (C-E:1), SSI=0.10/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES						
PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

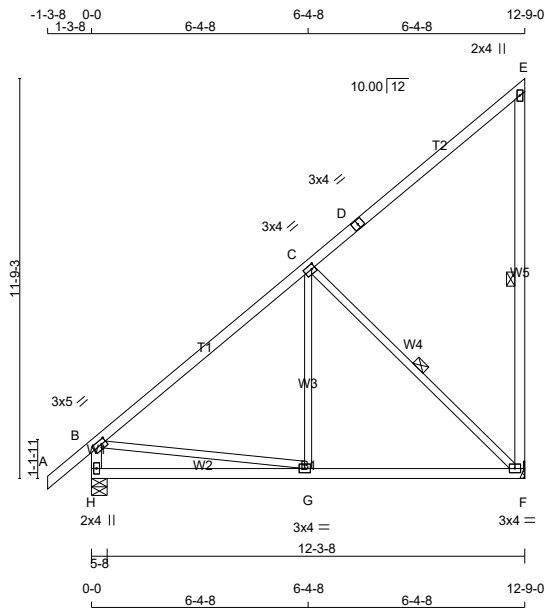
JSI GRIP= 0.32 (C) (INPUT = 0.90)
JSI METAL= 0.11 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
336324	H98		1	TRUSS DESC. JT 45147	E21104097

Alpa Roof Truss, Maple

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed Oct 13 14:52:15 2021 Page 1

ID:hd511hZTz bdeQoS817GKByhVxV-xQEL11C9EIwuw1AUaNnGcta8GvsfMKwLKHSZ qyTp6U



TOTAL WEIGHT = 3 X 66 = 197 lb

[M][F]

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
DRY: SEASONED LUMBER.				

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	3.0	5.0	1.50	1.75
C	TMWW-t	MT20	3.0	4.0	1.50	1.25
D	TS-t	MT20	3.0	4.0		
E	TMV+p	MT20	2.0	4.0		
F	BMVW1-t	MT20	3.0	4.0		
G	BMWW-t	MT20	3.0	4.0		
H	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQRD
	VERT	HORZ	DOWN	HORZ
JT	615	0	0	0
F	615	0	0	0
H	723	0	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	438	268 / 0	0 / 0	0 / 0	0 / 0	171 / 0	0 / 0
H	513	326 / 0	0 / 0	0 / 0	0 / 0	187 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-F, C-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 35	-78.0 -78.0	0.11 (1)	10.00	G-C	0 / 145	0.05 (4)
B-C	-480 / 0	-78.0 -78.0	0.43 (1)	6.25	C-F	-553 / 0	0.29 (1)
C-D	-39 / 0	-78.0 -78.0	0.42 (1)	6.25	B-G	0 / 402	0.09 (1)
D-E	-39 / 0	-78.0 -78.0	0.42 (1)	6.25			
F-E	-187 / 0	0.0 0.0	0.14 (1)	6.25			
H-B	-678 / 0	0.0 0.0	0.07 (1)	7.81			
H-G	0 / 0	-18.5 -18.5	0.21 (4)	10.00			
G-F	0 / 399	-18.5 -18.5	0.25 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	34.4	PSF	

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.42")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.42")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.43/1.00 (B-C:1), BC=0.25/1.00 (F-G:4),
WB=0.29/1.00 (C-F:1), SSI=0.19/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES					
PLATE GRIP(DRY)	SHEAR	SECTION			
(PSI)	(PLI)	(PLI)			
MAX MIN	MAX MIN	MAX MIN			
MT20	650	371	1747	788	1987

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.68 (C) (INPUT = 0.90)
JSI METAL= 0.20 (B) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

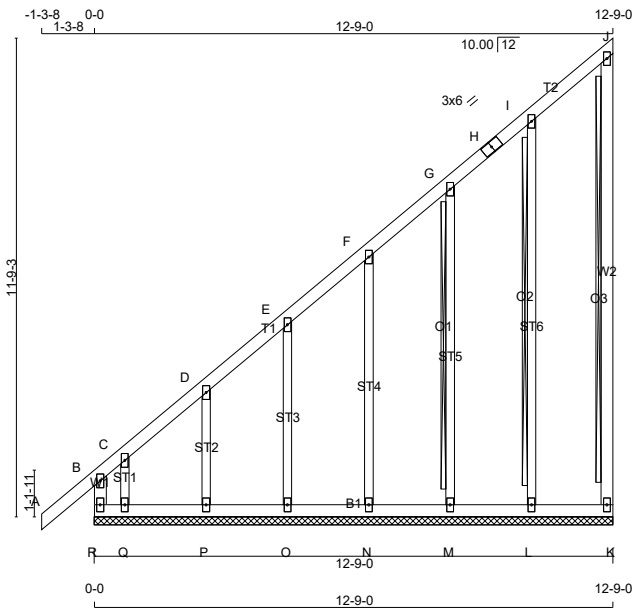


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
336324	H98G		1	TRUSS DESC. JT 45147	E21104098

Alpa Roof Truss, Maple

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Wed Oct 13 14:52:16 2021 Page 1

ID:hd511hZTz bdeQoS817GKByhVxV-QdokENDn b2IXAlq84IV847O1IFi5p1UZxC7X7yTp6T



TOTAL WEIGHT = 75 lb [M]

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER		DESCR.
R - B	2x4 DRY	No.2		SPF
A - H	2x4 DRY	No.2		SPF
H - J	2x4 DRY	No.2		SPF
K - J	2x4 DRY	No.2		SPF
R - K	2x4 DRY	No.2		SPF
ALL WEBS	2x3 DRY	No.2		SPF
ALL GABLE WEBS	2x3 DRY	No.2		SPF
DRY: SEASONED LUMBER.				
GABLE STUDS SPACED AT 2'-0" O.C.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	2.0	4.0	
C, D, E, F, G, I					
C	TMW+w	MT20	2.0	4.0	
H	TS-t	MT20	3.0	6.0	
J	TMV+p	MT20	2.0	4.0	
K	BMV1+p	MT20	2.0	4.0	
L, M, N, O, P, Q					
L	BMW1+w	MT20	2.0	4.0	
R	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT J-K, I-L

2x3 DRY SPF No.2 T-BRACE AT G-M

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			FR-TO		
R-B	-223 / 0	0.0 0.0	0.03 (1)	7.81	L-I	-169 / 0	0.13 (1)
A-B	0 / 35	-78.0 -78.0	0.11 (1)	10.00	M-G	-153 / 0	0.17 (1)
B-C	-70 / 0	-78.0 -78.0	0.09 (1)	6.25	N-F	-155 / 0	0.12 (1)
C-D	-15 / 0	-78.0 -78.0	0.04 (1)	6.25	O-E	-152 / 0	0.06 (1)
D-E	-14 / 0	-78.0 -78.0	0.04 (1)	6.25	P-D	-161 / 0	0.03 (1)
E-F	-9 / 0	-78.0 -78.0	0.04 (1)	10.00	Q-C	-19 / 0	0.00 (1)
F-G	-6 / 0	-78.0 -78.0	0.04 (1)	10.00			
G-H	-3 / 0	-78.0 -78.0	0.04 (1)	10.00			
H-I	-3 / 0	-78.0 -78.0	0.04 (1)	10.00			
I-J	-8 / 0	-78.0 -78.0	0.04 (1)	10.00			
K-J	-71 / 0	0.0 0.0	0.02 (1)	7.81			
R-Q	0 / 16	-18.5 -18.5	0.03 (1)	10.00			
Q-P	0 / 15	-18.5 -18.5	0.02 (4)	10.00			
P-O	0 / 10	-18.5 -18.5	0.02 (4)	10.00			
O-N	0 / 7	-18.5 -18.5	0.02 (4)	10.00			
N-M	0 / 4	-18.5 -18.5	0.02 (4)	10.00			
M-L	0 / 3	-18.5 -18.5	0.02 (4)	10.00			
L-K	0 / 1	-18.5 -18.5	0.02 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 21.0 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11/1.00 (A-B:1) , BC=0.03/1.00 (Q-R:1) , WB=0.17/1.00 (G-M:1) , SSI=0.06/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90)

JSI METAL= 0.13 (B) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



Alpa Roof Truss, Maple

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Oct 14 08:52:54 2021 Page 1



TOTAL WEIGHT = 42 lb



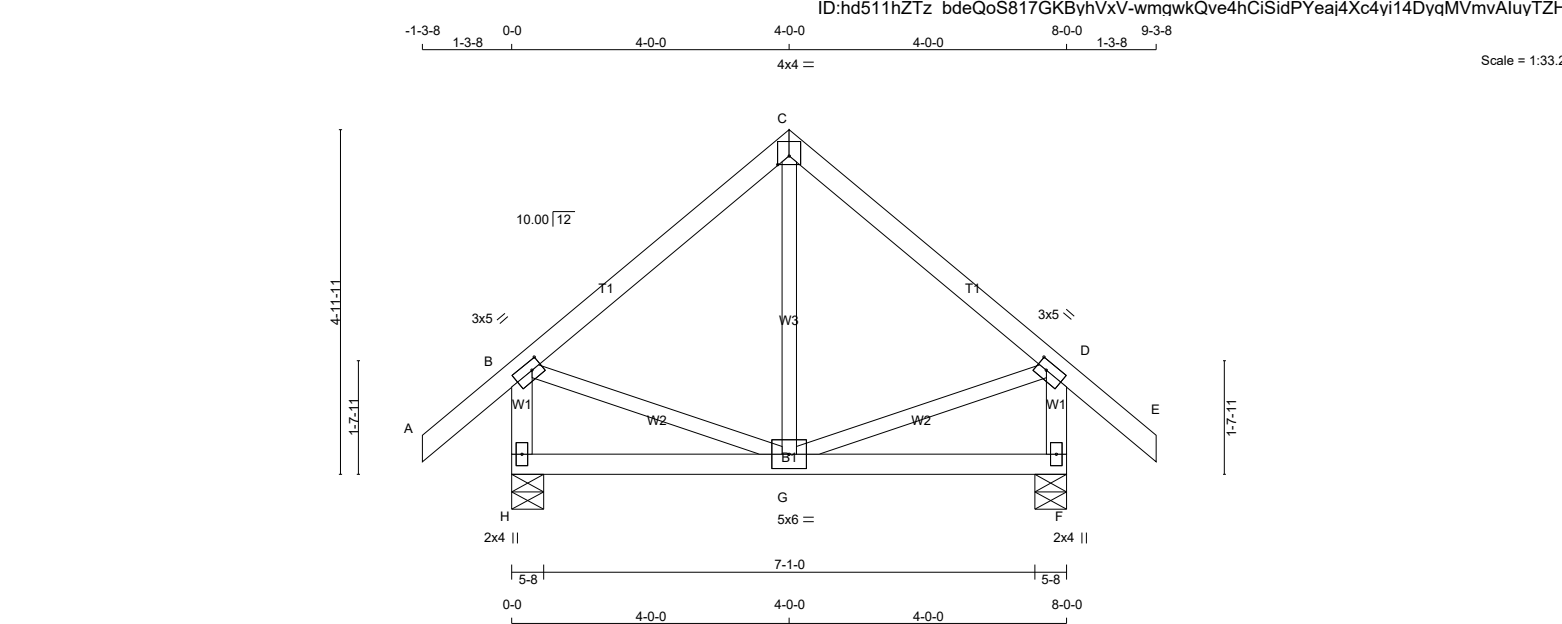
JSI GRIP= 0.44 (B) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
336335	H100		1	TRUSS DESC. JT 45147	E21104165

Alpa Roof Truss, Maple

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Oct 14 08:52:56 2021 Page 1

ID:hd511hZTz bdeQoS817GKByhVxV-wmgwkQve4hCiSidPYeaj4Xc4yi14DyqMVmvAluyTZHL



LUMBER

N. L. G. A. RULES

CHORDS SIZE LUMBER DESCR.

A - C 2x4 DRY No.2 SPF

C - E 2x4 DRY No.2 SPF

H - B 2x4 DRY No.2 SPF

F - D 2x4 DRY No.2 SPF

H - F 2x4 DRY No.2 SPF

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

DRY: SEASONED LUMBER.

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - E	2x4	DRY No.2	SPF
H - B	2x4	DRY No.2	SPF
F - D	2x4	DRY No.2	SPF
H - F	2x4	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	3.0	5.0	1.50	1.75
C	TTW-p	MT20	4.0	4.0	1.50	2.00
D	TMVW-t	MT20	3.0	5.0	1.50	1.75
F	BMV1+p	MT20	2.0	4.0		
G	BMVWWW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	2.0	4.0		

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX	MAX. MEMB. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX
FR-TO		FROM	TO			FR-TO		
A-B	0 / 35	-78.0	-78.0	0.11 (1)	10.00	G-C	-23 / 55	0.02 (4)
B-C	-226 / 0	-78.0	-78.0	0.16 (1)	6.25	B-G	0 / 183	0.04 (1)
C-D	-226 / 0	-78.0	-78.0	0.16 (1)	6.25	G-D	0 / 183	0.04 (1)
D-E	0 / 35	-78.0	-78.0	0.11 (1)	10.00			
H-B	-465 / 0	0.0	0.0	0.05 (1)	7.81			
F-D	-465 / 0	0.0	0.0	0.05 (1)	7.81			
H-G	0 / 0	-18.5	-18.5	0.08 (4)	10.00			
G-F	0 / 0	-18.5	-18.5	0.08 (4)	10.00			

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION			MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	494	0	494	0	0	5-8	1-8
F	494	0	494	0	0	5-8	1-8

UNFACTORED REACTIONS							
1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	350	226 / 0	0 / 0	0 / 0	0 / 0	124 / 0	0 / 0
F	350	226 / 0	0 / 0	0 / 0	0 / 0	124 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX	MAX. MEMB. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX
FR-TO		FROM	TO			FR-TO		
A-B	0 / 35	-78.0	-78.0	0.11 (1)	10.00	G-C	-23 / 55	0.02 (4)
B-C	-226 / 0	-78.0	-78.0	0.16 (1)	6.25	B-G	0 / 183	0.04 (1)
C-D	-226 / 0	-78.0	-78.0	0.16 (1)	6.25	G-D	0 / 183	0.04 (1)
D-E	0 / 35	-78.0	-78.0	0.11 (1)	10.00			
H-B	-465 / 0	0.0	0.0	0.05 (1)	7.81			
F-D	-465 / 0	0.0	0.0	0.05 (1)	7.81			
H-G	0 / 0	-18.5	-18.5	0.08 (4)	10.00			
G-F	0 / 0	-18.5	-18.5	0.08 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 21.0 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN./C/C

THIS DESIGN IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.27")

CALCULATED VERT. DEFL.(LL) = L/999 (0.00")

ALLOWABLE DEFL.(TL)= L/360 (0.27")

CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.16/1.00 (C-D:1), BC=0.08/1.00 (F-G:4), WB=0.04/1.00 (B-G:1), SSI=0.09/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES						
PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.39 (D) (INPUT = 0.90)

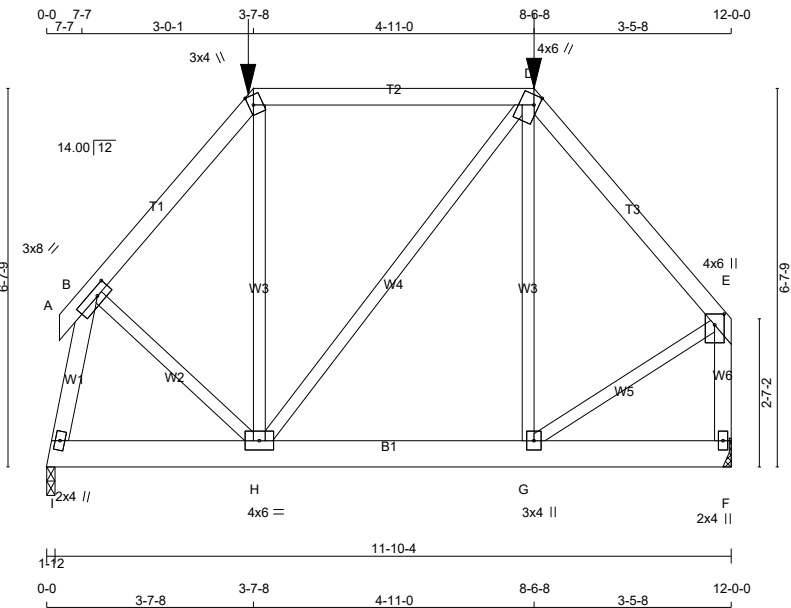
JSI METAL= 0.11 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
336335	H102A		1	TRUSS DESC. JT 45147	E21104166

Alpa Roof Truss, Maple

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Oct 14 08:52:57 2021 Page 1

ID:hd511hZTz bdeQoS817GKBvhVxV-OzElxmwGr?KZ4rCb5L5yck89x6NJyOvWjQeirKyTZHK



TOTAL WEIGHT = 67 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - F	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT				
B - H	2x3	DRY	No.2	SPF
H - C	2x3	DRY	No.2	SPF
H - D	2x3	DRY	No.2	SPF
G - D	2x3	DRY	No.2	SPF
G - E	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMWW-t	MT20	3.0	8.0 1.50 3.00
C	TTW+m	MT20	3.0	4.0 2.00 1.00
D	TTWW+m	MT20	4.0	6.0 2.00 1.00
E	TMVW+p	MT20	4.0	6.0 2.25 2.00
F	BMV1+p	MT20	2.0	4.0
G	BMWW+t	MT20	3.0	4.0
H	BMWWW-t	MT20	4.0	6.0
I	BMW1+w	MT20	2.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
	FACTORED		MAXIMUM FACTORED		INPUT		REQRD		
	GROSS REACTION		GROSS REACTION		BRG		BRG		
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX		
I	828	0	828	0	0	1-12	1-8		
F	817	0	817	0	0	MECHANICAL			

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS							
	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	590	360 / 0	0 / 0	0 / 0	0 / 0	231 / 0	0 / 0
F	583	354 / 0	0 / 0	0 / 0	0 / 0	229 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING									
TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM TO				FR-TO			
A - B	0 / 19	-78.0 -78.0	0.03 (1)	10.00	I - B	-803 / 0	0.12 (1)		
B - C	-556 / 0	-78.0 -78.0	0.13 (1)	6.25	B - H	0 / 296	0.07 (1)		
C - D	-358 / 0	-114.5 -114.5	0.53 (1)	6.25	H - C	-81 / 56	0.06 (1)		
D - E	-559 / 0	-78.0 -78.0	0.19 (1)	6.25	H - D	-4 / 0	0.00 (1)		
F - E	-779 / 0	0.0 0.0	0.11 (1)	7.81	G - D	-94 / 54	0.06 (1)		
					G - E	0 / 422	0.10 (1)		
I - H	0 / 145	-27.2 -27.2	0.06 (4)	10.00					
H - G	0 / 360	-27.2 -27.2	0.08 (4)	10.00					
G - F	0 / 0	-27.2 -27.2	0.05 (4)	10.00					

FACTORED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-7-8	-112	-112	---	FRONT	VERT	TOTAL	---	C1
D	8-6-8	-107	-107	---	FRONT	VERT	TOTAL	---	C1

DESIGN CRITERIA		
SPECIFIED LOADS:		
TOP CH.	LL = 21.0	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 34.4	PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 3-7-9
RIGHT SETBACK = 3-5-8
END SETBACK = 3-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.39")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.39")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.01")

CSI: TC=0.53/1.00 (C-D:1) , BC=0.08/1.00 (G-H:4) , WB=0.12/1.00 (B-I:1) , SSI=0.24/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (G) (INPUT = 0.90)
JSI METAL= 0.22 (E) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



Alpa Roof Truss, Maple

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Oct 14 08:52:59 2021 Page 1



TOTAL WEIGHT = 67 lb

DRY: SEASONED LUMBER.

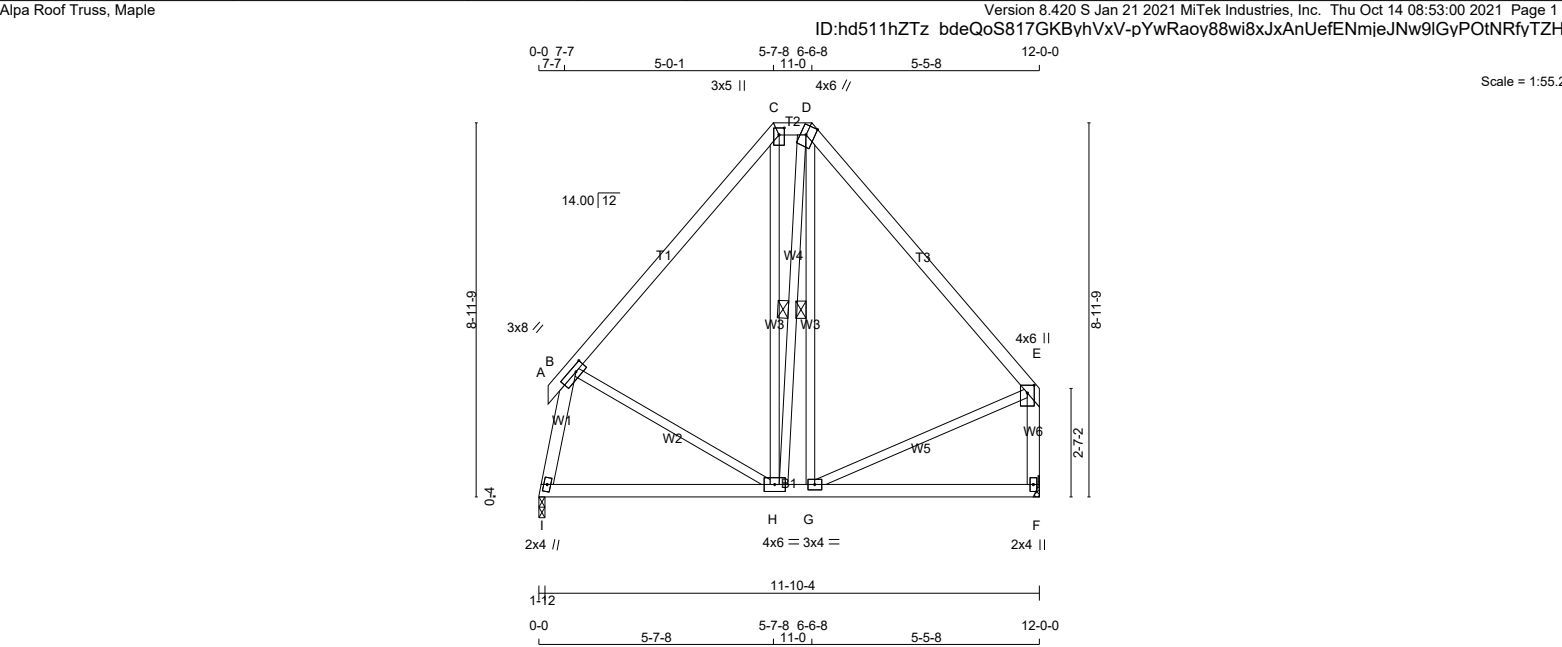
Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

TOTAL LOAD CASES: (4)

FACTORED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
B	3-5-8	-107	-107	---	FRONT	VERT	TOTAL	---	C1
C	8-4-8	-116	-116	---	FRONT	VERT	TOTAL	---	C1

JSI GRIP= 0.48 (C) (INPUT = 0.90)
JSI METAL= 0.22 (A) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
336335	H103A		1	TRUSS DESC. JT 45147	E21104168



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR.					DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS							DESIGN CRITERIA			[M][F]		
A - C	2x4	DRY	No.2	SPF	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG	SPECIFIED LOADS:						
C - D	2x4	DRY	No.2	SPF	JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	TOP CH.	LL = 21.0 PSF			
D - E	2x4	DRY	No.2	SPF	I	574	0	574	0	0	1-12	1-8	DL =	6.0 PSF			
F - E	2x4	DRY	No.2	SPF	F	570	0	570	0	0	MECHANICAL		BOT CH.	LL = 0.0 PSF			
I - F	2x4	DRY	No.2	SPF											DL =	7.4 PSF	
					A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.										TOTAL LOAD =	34.4 PSF	
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF											SPACING = 24.0 IN./C		
B - H	2x3	DRY	No.2	SPF											LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM		
H - C	2x3	DRY	No.2	SPF											THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015		
H - D	2x3	DRY	No.2	SPF													
G - D	2x3	DRY	No.2	SPF													
G - E	2x3	DRY	No.2	SPF													
DRY: SEASONED LUMBER.																	
					UNFACTORED REACTIONS												
					1ST LCASE		MAX./MIN. COMPONENT REACTIONS										
					JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL					
					I	409	250 / 0	0 / 0	0 / 0	0 / 0	159 / 0	0 / 0					
					F	407	248 / 0	0 / 0	0 / 0	0 / 0	158 / 0	0 / 0					

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMWW-t	MT20	3.0	8.0	1.50 3.00
C	TTW+p	MT20	3.0	5.0	2.00 1.50
D	TTWW+m	MT20	4.0	6.0	Edge 2.50
E	TMVW+p	MT20	4.0	6.0	2.25 2.00
F	BMV1+p	MT20	2.0	4.0	
G	BMWW-t	MT20	3.0	4.0	
H	BMWWW-t	MT20	4.0	6.0	
I	BMW1+w	MT20	2.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

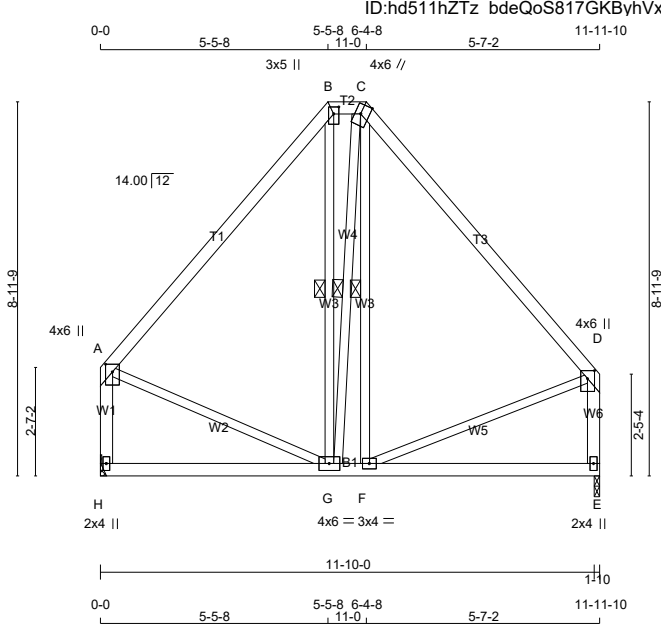


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
336335	H103S		1	TRUSS DESC. JT 45147	E21104169

Alpa Roof Truss, Maple

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Oct 14 08:53:02 2021 Page 1

ID:hd511hZTz bdeQoS817GKBvhVxV-lw2B T OqXyrAd4Yuuq8Jos2u73LdqBFtiMUWYyTZHF



LUMBER N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW+p	MT20	4.0	6.0	2.25 2.00
B	TTW+p	MT20	3.0	5.0	2.00 1.50
C	TTWW+m	MT20	4.0	6.0	Edge 2.50
D	TMVW+p	MT20	4.0	6.0	2.25 2.00
E	BMV1+p	MT20	2.0	4.0	
F	BMWW-t	MT20	3.0	4.0	
G	BMWWW-t	MT20	4.0	6.0	
H	BMV1+p	MT20	2.0	4.0	
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.					

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER							
BEARINGS							
	FACTORED		MAXIMUM FACTORED		INPUT	REQRD	
	GROSS REACTION		GROSS REACTION		BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	577	0	577	0	0	MECHANICAL	
E	577	0	577	0	0	1-10	1-8
A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 1-8.							

UNFACTORED REACTIONS							
1ST LCASE	MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	412	251 / 0	0 / 0	0 / 0	0 / 0	160 / 0	0 / 0
E	412	251 / 0	0 / 0	0 / 0	0 / 0	160 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF B-G, C-G, C-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING							
TOTAL LOAD CASES: (4)							
CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX	MAX. UNBRAC	MEMB.	FORCE (LBS)	MAX
FR-TO		FROM TO	CSI (LC)	LENGTH FR-TO			CSI (LC)
A-B	-314 / 0	-78.0	-78.0 0.31 (1)	6.25	G-B	-8 / 46	0.02 (4)
B-C	-204 / 0	-78.0	-78.0 0.01 (1)	6.25	G-C	-10 / 0	0.01 (1)
C-D	-315 / 0	-78.0	-78.0 0.32 (1)	6.25	F-C	-2 / 46	0.02 (4)
H-A	-536 / 0	0.0	0.0 0.07 (1)	7.81	A-G	0 / 220	0.05 (1)
E-D	-535 / 0	0.0	0.0 0.07 (1)	7.81	F-D	0 / 219	0.05 (1)
H-G	0 / 0	-18.5	-18.5 0.13 (4)	10.00			
G-F	0 / 205	-18.5	-18.5 0.16 (4)	10.00			
F-E	0 / 0	-18.5	-18.5 0.13 (4)	10.00			

DESIGN CRITERIA			
SPECIFIED LOADS:			
TOP CH.	LL = 21.0	PSF	
	DL = 6.0	PSF	
BOT CH.	LL = 0.0	PSF	
	DL = 7.4	PSF	
TOTAL LOAD	= 34.4	PSF	
SPACING = 24.0 IN./C			

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.40")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.40")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.03")

CSI: TC=0.32/1.00 (C-D:1) , BC=0.16/1.00 (F-G:4) , WB=0.05/1.00 (A-G:1) , SSI=0.11/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.31 (F) (INPUT = 0.90)
JSI METAL= 0.16 (D) (INPUT = 1.00)

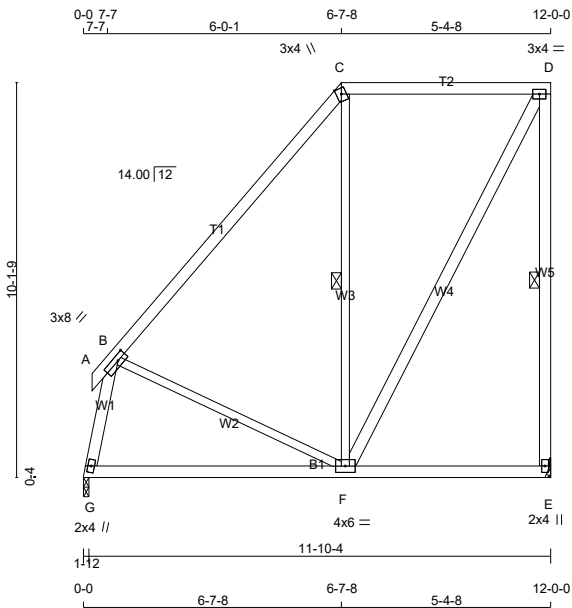
LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
336335	H104A		1	TRUSS DESC. JT 45147	E21104170

Alpa Roof Truss, Maple

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Oct 14 08:53:03 2021 Page 1

ID:hd511hZTz bdeQoS817GKByhVxV-D7cZCp?1Rr4iomfScCNs?ODHWOUm5hO6M512 yTZHE



TOTAL WEIGHT = 71 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - D	2x4	DRY No.2	SPF
E - D	2x4	DRY No.2	SPF
G - E	2x4	DRY No.2	SPF
ALL WEBS EXCEPT	2x4	DRY No.2	SPF
B - F	2x3	DRY No.2	SPF
F - C	2x3	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMWW-t	MT20	3.0	8.0	1.50 3.00
C	TTW+m	MT20	3.0	4.0	Edge
D	TMVW-t	MT20	3.0	4.0	
E	BMV1+p	MT20	2.0	4.0	
F	BMVWW-t	MT20	4.0	6.0	
G	BMW1+w	MT20	2.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
G	574	0	574	0	1-12	1-8
E	570	0	570	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	409	250 / 0	0 / 0	0 / 0	0 / 0	159 / 0	0 / 0
E	407	248 / 0	0 / 0	0 / 0	0 / 0	158 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-E, C-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)		MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
		FROM	TO				
A-B	0 / 19	-78.0	-78.0 0.02 (1)	10.00	G-B	-535 / 0	0.08 (1)
B-C	-275 / 0	-78.0	-78.0 0.35 (1)	6.25	B-F	0 / 87	0.02 (1)
C-D	-176 / 0	-78.0	-78.0 0.29 (1)	6.25	F-C	-230 / 0	0.16 (1)
E-D	-537 / 0	0.0	0.0 0.27 (1)	6.25	F-D	0 / 372	0.06 (1)
G-F	0 / 100	-18.5	-18.5 0.20 (4)	10.00			
F-E	0 / 0	-18.5	-18.5 0.19 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	34.4	PSF	

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.39")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.39")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.35/1.00 (B-C:1) , BC=0.20/1.00 (F-G:4) , WB=0.16/1.00 (C-F:1) , SSI=0.16/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES					
PLATE	GRIP(DRY)	SHEAR	SECTION		
	(PSI)	(PLI)	(PLI)		
	MAX	MIN	MAX	MIN	MAX
MT20	650	371	1747	788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

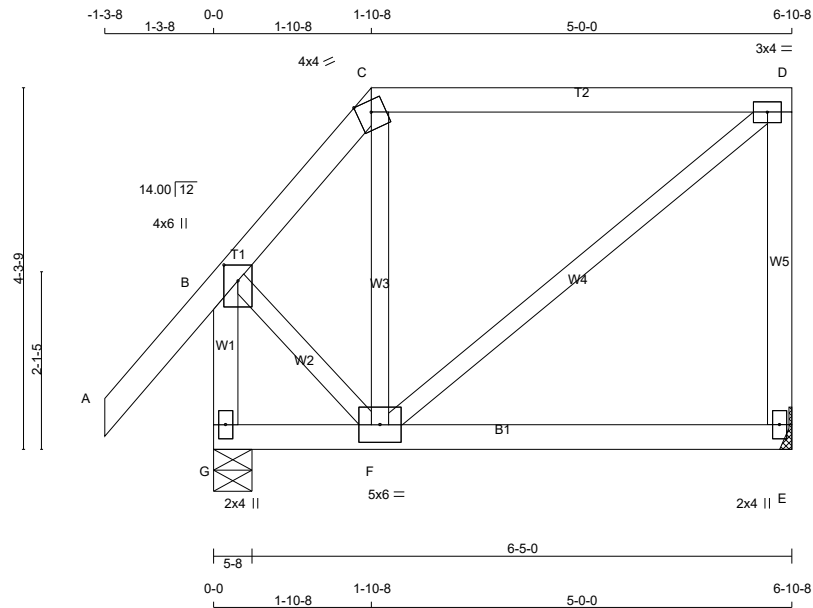
JSI GRIP= 0.61 (D) (INPUT = 0.90)
JSI METAL= 0.12 (D) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



Alpa Roof Truss, Maple

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Oct 14 08:53:05 2021 Page 1
ID:hd511hZTz bdeQoS817GKByhVxV-9VjKdV0HySKQ14p7Z1ErXQUZzK6?q0BhZga87tyTZHC



TOTAL WEIGHT = 35 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF

ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
--------------------	-----	-----	------	-----

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.25	2.00
C	TTW-m	MT20	4.0	4.0	Edge	
D	TMVW-t	MT20	3.0	4.0		
E	BMV1+p	MT20	2.0	4.0		
F	BMWWW-t	MT20	5.0	6.0		
G	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
BEARINGS

	FACTORED		MAXIMUM FACTORED			INPUT	REQRD
	GROSS REACTION		GROSS REACTION			BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	320	0	320	0	0	MECHANICAL	
G	452	0	452	0	0	5-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	229	138 / 0	0 / 0	0 / 0	0 / 0	90 / 0	0 / 0
G	320	209 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX FACTORED CSI (LC)		
FR-TO		FROM TO			FR-TO				
A - B	0 / 42	-78.0	-78.0 0.11 (1)	10.00	F - C	-96 / 13	0.03 (1)		
B - C	-204 / 0	-78.0	-78.0 0.11 (1)	6.25	F - D	0 / 140	0.03 (1)		
C - D	-109 / 0	-78.0	-78.0 0.33 (1)	6.25	B - F	0 / 148	0.03 (1)		
E - D	-283 / 0	0.0	0.0 0.08 (1)	7.81					
G - B	-453 / 0	0.0	0.0 0.05 (1)	7.81					
G - F	0 / 0	-18.5	-18.5 0.09 (4)	10.00					
F - E	0 / 0	-18.5	-18.5 0.09 (4)	10.00					

LATERAL BRACE(S) SHOWN SHALL BE
2X4 SPF#2



DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	21.0	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF

TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.23")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.33/1.00 (C-D:1), BC=0.09/1.00 (F-G:4), WB=0.03/1.00 (B-F:1), SSI=0.15/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

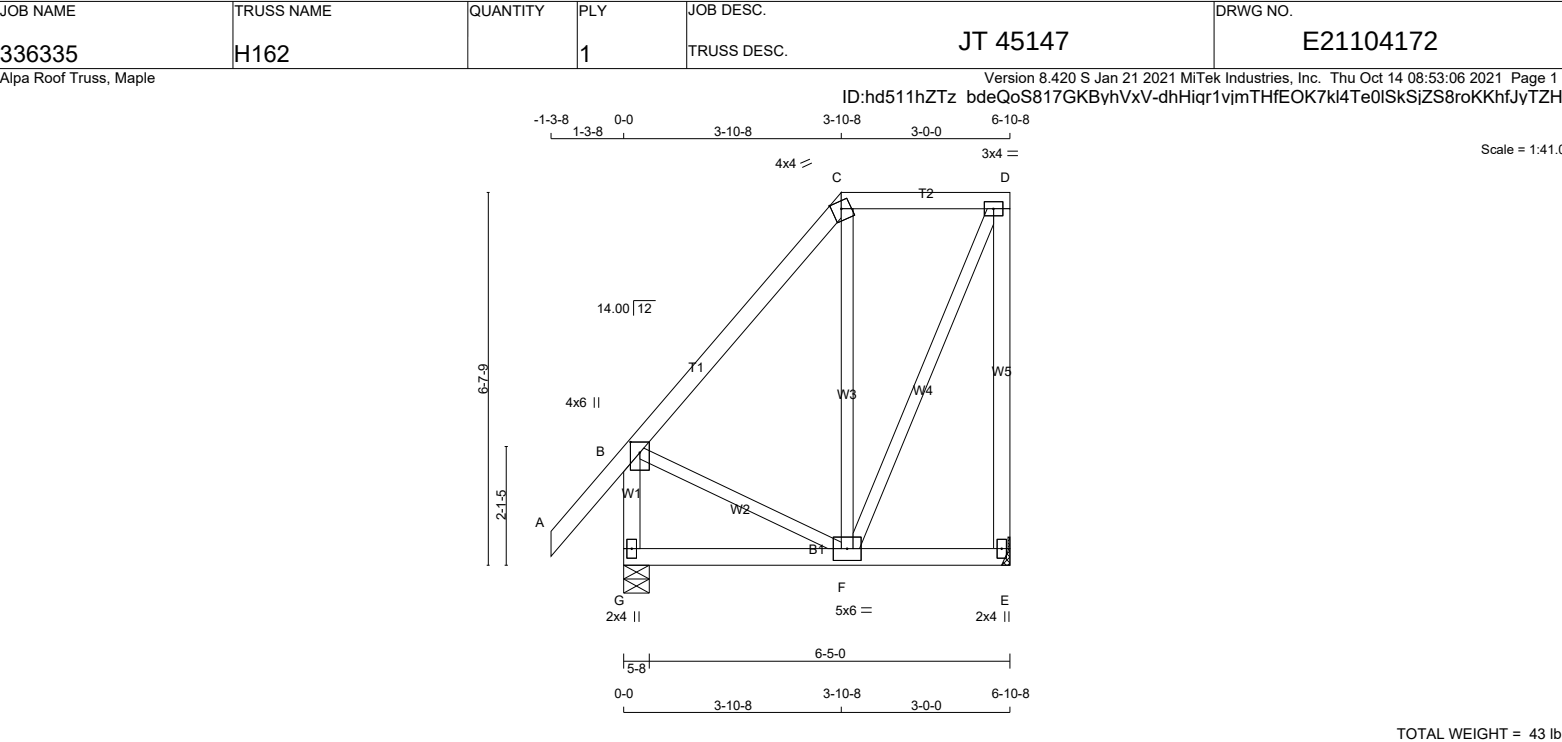
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP(DRY)		SHEAR		SECTION
	(PSI)		(PLI)		(PLI)
	MAX	MIN	MAX	MIN	MAX MIN
MT20	650	371	1747	788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (D) (INPUT = 0.90)
JSI METAL= 0.11 (B) (INPUT = 1.00)



LUMBER

N. L. G. A. RULES

CHORDS

SIZE

DRY

No.2

SPF

ALL WEBS

2x3

DRY

No.2

SPF

EXCEPT

DESCR.

SPF

SPF

SPF

SPF

SPF

DRY: SEASONED LUMBER.

PLATES

(table is in inches)

JT

TYPE

PLATES

W

LEN

Y

X

B

TMVW+p

MT20

4.0

6.0

2.25

2.00

C

TTW-m

MT20

4.0

4.0

Edge

D

TMVW-t

MT20

3.0

4.0

E

BMV1+p

MT20

2.0

4.0

F

BMVWW-t

MT20

5.0

6.0

G

BMV1+p

MT20

2.0

4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

LICENSED PROFESSIONAL ENGINEER

Y. WIDYA

100225448

10/14/2021

PROVINCE OF ONTARIO

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED GROSS REACTION

MAXIMUM FACTORED GROSS REACTION

INPUT BRG

REQRD BRG

JT

VERT

HORZ

DOWN

HORZ

UPLIFT

IN-SX

IN-SX

E

332

0

332

0

0

MECHANICAL

G

441

0

441

0

0

5-8

1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

1ST LCASE

MAX./MIN. COMPONENT REACTIONS

JT

COMBINED

SNOW

LIVE

PERM.LIVE

WIND

DEAD

SOIL

E

236

144 / 0

0 / 0

0 / 0

0 / 0

92 / 0

0 / 0

G

312

203 / 0

0 / 0

0 / 0

0 / 0

109 / 0

0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MAX. FACTORED FORCE (LBS)

FACTORED VERT. LOAD (PLF)

LC1

MAX

CS1 (LC)

UNBRAC LENGTH

FR-TO

MEMB.

A-B

0 / 42

-78.0

-78.0

0.11 (1)

10.00

F-C

-160 / 0

0.12 (1)

B-C

-143 / 0

-78.0

-78.0

0.15 (1)

6.25

F-D

0 / 217

0.05 (1)

C-D

-90 / 0

-78.0

-78.0

0.09 (1)

6.25

B-F

0 / 101

0.02 (1)

E-D

-313 / 0

0.0

0.0

0.29 (1)

7.81

G-B

-410 / 0

0.0

0.0

0.05 (1)

7.81

G-F

0 / 0

-18.5

-18.5

0.06 (4)

10.00

F-E

0 / 0

-18.5

-18.5

0.06 (4)

10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.

LL

=

21.0

PSF

DL

=

6.0

PSF

BOT CH.

LL

=

0.0

PSF

DL

=

7.4

PSF

TOTAL LOAD

=

34.4

PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.23")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.23")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.01")

CSI: TC=0.29/1.00 (D-E:1) , BC=0.06/1.00 (F-G:4) , WB=0.12/1.00 (C-F:1) , SSI=0.09/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE GRIP(DRY) (PSI)

SHEAR (PLI)

SECTION (PLI)

MAX

MIN

MAX

MIN

MAX

MIN

MT20

650

371

1747

788

1987

1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

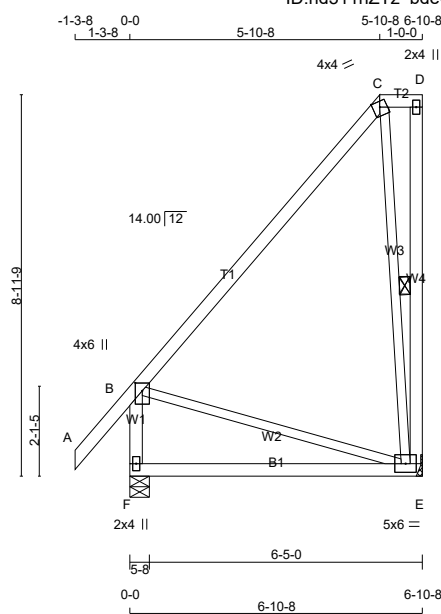
JSI GRIP= 0.47 (D) (INPUT = 0.90)
JSI METAL= 0.11 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
336335	H163		1	TRUSS DESC. JT 45147	E21104173

Alpa Roof Truss, Maple

ID:hd511hZTz bdeQoS817GKBhVxV-5ur42B2XU4b8HOzWhSGJ0rZvJ8kxlvF 0 3FBlyTZHA

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Oct 14 08:53:07 2021 Page 1



Scale = 1:54.2

TOTAL WEIGHT = 45 lb
[M][F]

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER		DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
F - B	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
DRY: SEASONED LUMBER.				

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.25	2.00
C	TTW-m	MT20	4.0	4.0	Edge	
D	TMV+p	MT20	2.0	4.0		
E	BMVWW1-t	MT20	5.0	6.0		
F	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
JT	332	0	332	0	0	0
E	332	0	332	0	0	0
F	441	0	441	0	5-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	236	144 / 0	0 / 0	0 / 0	0 / 0	92 / 0	0 / 0
E	236	144 / 0	0 / 0	0 / 0	0 / 0	92 / 0	0 / 0
F	312	203 / 0	0 / 0	0 / 0	0 / 0	109 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-E, C-E.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MAX. UNBRAC LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO					
A-B	0 / 42	-78.0 -78.0	0.11 (1)	10.00	C-E	-238 / 0	0.12 (1)
B-C	-42 / 0	-78.0 -78.0	0.34 (1)	6.25	B-E	0 / 28	0.01 (1)
C-D	0 / 0	-78.0 -78.0	0.01 (1)	10.00			
E-D	-39 / 0	0.0 0.0	0.01 (1)	6.25			
F-B	-377 / 0	0.0 0.0	0.04 (1)	7.81			
F-E	0 / 0	-18.5 -18.5	0.26 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCS 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.23")
CALCULATED VERT. DEFL.(TL)= L/ 819 (0.10")

CSI: TC=0.34/1.00 (B-C:1) , BC=0.26/1.00 (E-F:4) ,
WB=0.12/1.00 (C-E:1) , SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES			
PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (B) (INPUT = 0.90)
JSI METAL= 0.13 (C) (INPUT = 1.00)

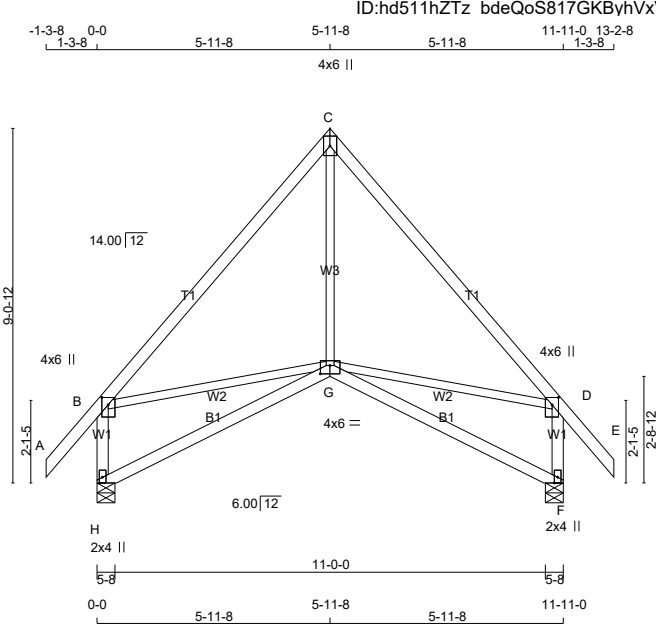
LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
336335	H200		1	TRUSS DESC. JT 45147	E21104174

Alpa Roof Truss, Maple

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Oct 14 08:53:09 2021 Page 1



Scale = 1:58.9

TOTAL WEIGHT = 60 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - C	2x4	DRY 1650F 1.5E	SPF	
C - E	2x4	DRY 1650F 1.5E	SPF	
H - B	2x4	DRY No.2	SPF	
F - D	2x4	DRY No.2	SPF	
H - G	2x4	DRY No.2	SPF	
G - F	2x4	DRY No.2	SPF	
ALL WEBS	2x3	DRY No.2	SPF	
EXCEPT				
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	4.0	6.0	2.25 2.00
C	TTW+p	MT20	4.0	6.0	
D	TMVW+p	MT20	4.0	6.0	2.25 2.00
F	BMV1+p	MT20	2.0	4.0	Edge
G	BBWW+p	MT20	4.0	6.0	2.50 3.00
H	BMV1+p	MT20	2.0	4.0	Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	684	0	684	0	5-8	1-8	
F	684	0	684	0	5-8	1-8	

UNFACTORED REACTIONS							
JT	1ST LCASE	MAX.	MIN.	COMPONENT	REACTIONS		
H	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	486	309 / 0	0 / 0	0 / 0	0 / 0	177 / 0	0 / 0
F	486	309 / 0	0 / 0	0 / 0	0 / 0	177 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 42	-78.0	-78.0 0.09 (1)	G-C	0 / 208	0.06 (4)	
B-C	-443 / 0	-78.0	-78.0 0.30 (1)	B-G	0 / 292	0.07 (1)	
C-D	-443 / 0	-78.0	-78.0 0.30 (1)	G-D	0 / 292	0.07 (1)	
D-E	0 / 42	-78.0	-78.0 0.09 (1)				
H-B	-629 / 0	0.0	0.0 0.08 (1)				
F-D	-629 / 0	0.0	0.0 0.08 (1)				
H-G	0 / 0	-18.5	-18.5 0.19 (4)				
G-F	0 / 0	-18.5	-18.5 0.19 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.40")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.40")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.30/1.00 (C-D:1) , BC=0.19/1.00 (G-H:4) ,
WB=0.07/1.00 (D-G:1) , SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.48 (G) (INPUT = 0.90)
JSI METAL= 0.26 (H) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



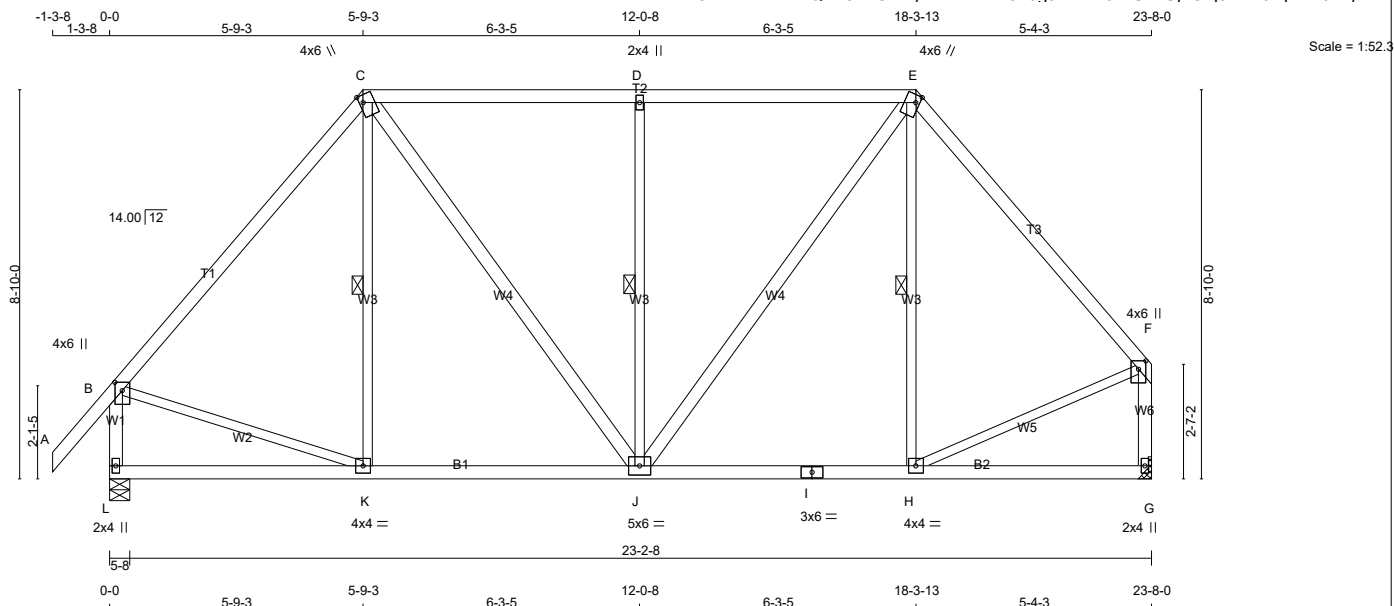
Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Oct 14 08:53:10 2021 Page 1



JSI GRIP= 0.80 (B) (INPUT = 0.90)
JSI METAL= 0.53 (B) (INPUT = 1.00)

Alpa Roof Truss, Maple

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Oct 14 08:53:12 2021 Page 1
ID:hd511hZTz bdeQoS817GKBvhVxV-Srez5u6gJcDRN9rTU?sUjvGig9SYz6HIAFn0szyTZH5



TOTAL WEIGHT = 2 X 121 = 242 lb

N. L. G. A. RULES				
CHORDS	SIZE		LUMBER	DESCR
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - J	2x4	DRY	No.2	SPF
J - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

J	T	TYPE	PLATES	W	LEN	Y	X
B	T	TMVw+p	MT20	4.0	6.0	2.25	2.00
C	D	TTVWw+m	MT20	4.0	6.0	2.00	1.00
B	C	TMVw	MT20	2.0	4.0		
E	F	TTVWw+m	MT20	4.0	6.0	2.00	1.00
F	G	TMVw+p	MT20	4.0	6.0	2.25	2.00
G	H	BMV1+p	MT20	2.0	4.0		
H	I	BMVWw-t	MT20	4.0	4.0		
I	J	BS-t	MT20	3.0	6.0		
J	K	BMVWw-t	MT20	5.0	6.0		
K	L	BMVW-t	MT20	4.0	4.0		
L		BMV1+p	MT20	2.0	4.0		

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQ'D BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
L	1297	0	1297	0	0	5-8	1-8
G	1189	0	1189	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 1-8.

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	927	556 / 0	0 / 0	0 / 0	0 / 0	371 / 0	0 / 0
G	852	497 / 0	0 / 0	0 / 0	0 / 0	355 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

FOR SECTION C-E, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.60 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-K, D-J, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MAX. FACTORED		FACTORED			MAX. FACTORED				
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC	MEMB.	FORCE (LBS)	MAX CSI (LC)		
FR-TO		FROM TO		LENGTH	FR-TO				
A-B	0 / 42	-78.0	-78.0 0.11 (1)	10.00	K-C	-60 / 60	0.03 (1)		
B-C	-965 / 0	-78.0	-78.0 0.55 (1)	5.60	C-J	0 / 449	0.07 (1)		
C-D	-892 / 0	-85.5	-85.5 0.61 (1)	2.00	J-D	-657 / 0	0.33 (1)		
D-E	-892 / 0	-85.5	-85.5 0.61 (1)	2.00	J-E	0 / 502	0.08 (1)		
E-F	-918 / 0	-78.0	-78.0 0.46 (1)	5.84	H-E	-122 / 30	0.06 (1)		
L-B	-1254 / 0	0.0	0.0 0.15 (1)	7.18	B-K	0 / 653	0.15 (1)		
G-F	-1151 / 0	0.0	0.0 0.16 (1)	7.42	H-F	0 / 643	0.14 (1)		
L-K	0 / 0	-18.5	-18.5 0.16 (4)	10.00					
K-J	0 / 625	-18.5	-18.5 0.23 (4)	10.00					
J-I	0 / 594	-18.5	-18.5 0.21 (4)	10.00					
I-H	0 / 594	-18.5	-18.5 0.21 (4)	10.00					
H-G	0 / 0	-18.5	-18.5 0.15 (4)	10.00					

LATERAL BRACE(S) SHOWN SHALL BE
2X4 SPF#2



SPECIFIED LOADS:				
TOP	CH.	LL =	21.0	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON
PIGGYBACK TRUSS WITH SLOPES OF 6.00/12
AND -6.00/12 AND RESPECTIVE HEEL HEIGHTS
OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD
LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.79")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.79")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.61/1.00 (D-E:1), BC=0.23/1.00 (J-K:4),
WB=0.33/1.00 (D-J:1), SSI=0.26/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES					
PLATE	GRIP(DRY)		SHEAR		SECTION
	(PSI)		(PLI)		(PLI)
	MAX	MIN	MAX	MIN	MAX MIN
MT20	650	371	1747	788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

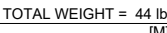
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (K) (INPUT = 0.90)
JSI METAL= 0.41 (B) (INPUT = 1.00)



Alpa Roof Truss, Maple

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Oct 14 08:53:15 2021 Page 1
ID:hd511hZTz bdeQoS817GKByhVxV-sQK6jw8YcXb?Eda297PBLXulsMW0AYL9sD?gTlyTZH2



DESCR.
SPF
SPF
SPF
SPF

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

CHORDS					WEBS				
MEMB.	MAX. FACTORED	FACTORED			MEMB.	MAX. FACTORED	FACTORED		
	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	LC1 MAX		FORCE (LBS)	LC1 MAX	LC1 MAX	
FR-TO		FROM	TO	LENGTH	FR-TO				
A-B	0 / 10	-78.0	-78.0	0.01 (1)	10.00	H-C	-119 / 0	0.04 (1)	
B-J	-38 / 26	-78.0	-78.0	0.08 (1)	6.25	C-G	-31 / 0	0.02 (1)	
J-C	-201 / 0	-78.0	-78.0	0.13 (1)	6.25	G-D	-165 / 0	0.05 (1)	
C-D	-98 / 0	-78.0	-78.0	0.24 (1)	6.25	I-J	-403 / 0	0.00 (1)	
D-L	-170 / 0	-78.0	-78.0	0.13 (1)	6.25	K-L	-410 / 0	0.00 (1)	
L-E	-31 / 64	-78.0	-78.0	0.08 (1)	6.25				
E-F	0 / 10	-78.0	-78.0	0.01 (1)	10.00				
B-I	0 / 123	-18.5	-18.5	0.12 (1)	10.00				
I-H	0 / 123	-18.5	-18.5	0.12 (1)	10.00				
H-G	0 / 120	-18.5	-18.5	0.08 (4)	10.00				
G-K	0 / 102	-18.5	-18.5	0.11 (1)	10.00				
K-E	0 / 102	-18.5	-18.5	0.11 (1)	10.00				

JSI GRIP= 0.40 (B) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)

Alpa Roof Truss, Maple

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Oct 14 08:53:17 2021 Page 1



TOTAL WEIGHT = 40 lb

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.50
C	TTW-p	MT20	4.0	4.0		
D	TMB1-I	MT20	3.0	4.0	1.50	2.50
F	BMW1+w	MT20	2.0	4.0		

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
B	485	0	485	0	0	11-6-7	1-8
D	485	0	485	0	0	11-6-7	1-8
F	203	0	203	0	0	11-6-7	1-8

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	342	230 / 0	0 / 0	0 / 0	0 / 0	112 / 0	0 / 0
D	342	230 / 0	0 / 0	0 / 0	0 / 0	112 / 0	0 / 0
F	151	56 / 0	0 / 0	0 / 0	0 / 0	95 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED			
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 10	-78.0	-78.0 0.01 (1)	10.00	F-C	0 / 32	0.01 (4)
B-H	0 / 494	-78.0	-78.0 0.31 (1)	10.00	G-H	-1130 / 0	0.00 (1)
H-C	-292 / 0	-78.0	-78.0 0.32 (1)	6.25	I-J	-1130 / 0	0.00 (1)
C-J	-292 / 0	-78.0	-78.0 0.32 (1)	6.25			
J-D	0 / 494	-78.0	-78.0 0.31 (1)	10.00			
D-E	0 / 10	-78.0	-78.0 0.01 (1)	10.00			
B-G	0 / 175	-18.5	-18.5 0.29 (1)	10.00			
G-F	0 / 175	-18.5	-18.5 0.29 (1)	10.00			
F-I	0 / 175	-18.5	-18.5 0.29 (1)	10.00			
I-D	0 / 175	-18.5	-18.5 0.29 (1)	10.00			

SPECIFIED LOADS:

TOP	CH.	LL	=	21.0	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.4	PSF
TOTAL LOAD				=	34.4 PSF

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.32/1.00 (C-J:1), BC=0.29/1.00 (F-I:1),
WB=0.01/1.00 (C-F:4), SSI=0.87/1.00 (D-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

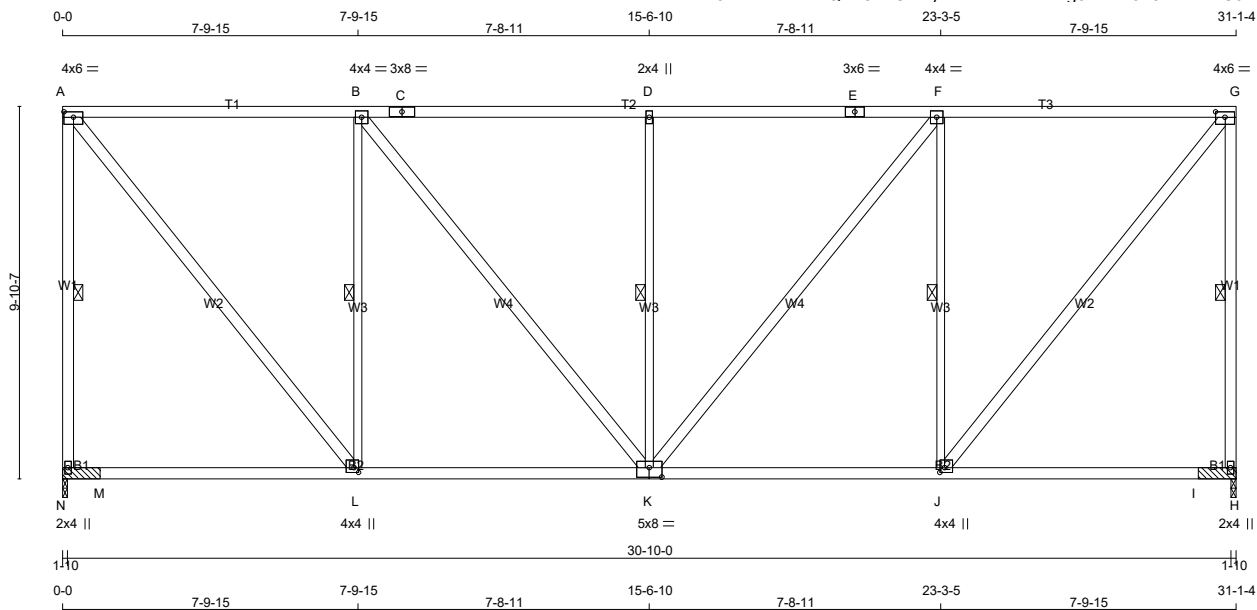
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.59 (D) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)



TOTAL WEIGHT = 2 X 171 = 343 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE		LUMBER	DESCR.
N - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	1650F 1.5E	SPF
C - E	2x4	DRY	1650F 1.5E	SPF
E - G	2x4	DRY	1650F 1.5E	SPF
H - G	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
L - B	2x3	DRY	No.2	SPF
K - D	2x3	DRY	No.2	SPF
J - F	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
A	TMWV-t	MT20	4.0	6.0	1.75	3.00
B	BMVW-t	MT20	4.0	4.0		
C	TS-t	MT20	3.0	8.0		
D	TMW+ w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWV-t	MT20	4.0	4.0		
G	TMVW-t	MT20	4.0	6.0	1.75	3.00
H	BMV1+p	MT20	2.0	4.0		
J	BMWV+t	MT20	4.0	4.0	1.50	1.50
K	BSWVWV-I	MT20	5.0	8.0	3.00	4.00
L	BMWV+t	MT20	4.0	4.0	1.50	1.50
N	BMV1+p	MT20	2.0	4.0		

LATERAL BRACE(S) SHOWN SHALL BE
2X4 SPF#2



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS							
	FACTORED		MAXIMUM FACTORED			INPUT	REQRD
	GROSS REACTION		GROSS REACTION			BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
N	1617	0	1617	0	0	1-10	1-10 & BLOCK
H	1617	0	1617	0	0	1-10	1-10 & BLOCK

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1163	653 / 0	0 / 0	0 / 0	0 / 0	510 / 0	0 / 0
H	1163	653 / 0	0 / 0	0 / 0	0 / 0	510 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

2x4 DRY SPF No.2 BEARING BLOCK 12" LONG AT JT. N ATTACHED TO FRONT SIDE WITH 2 ROWS OF (0.122"x3") SPIRAL NAILS SPACED 3" C.C. 8 NAILS TOTAL.

2x4 DRY SPF No.2 BEARING BLOCK 12" LONG AT JT. H ATTACHED TO FRONT SIDE WITH 2 ROWS OF (0.122"x3") SPIRAL NAILS SPACED 3" C.C. 8 NAILS TOTAL.

BRACING

FOR SECTION A-G, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 0.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF A-N, G-H, B-L, D-K, F-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MAX. FACTORED		FACTORED			MAX.		MAX. FACTORED		
MEMB.	FORCE	VERT. LOAD	MAX	MAX.	MEMB.	FORCE	MAX		
	(LBS)	(PLF)	CSI (LC)	UNBRAC		(LBS)	CSI (LC)		
FR-TO		FROM	TO	LENGTH	FR-TO				
N-A	-1562 / 0	0.0	0.0	0.73 (1)	5.27	A-L	0 / 1676	0.27 (1)	
A-B	-1060 / 0	-85.5	-85.5	0.71 (1)	2.00	L-B	-1119 / 0	0.74 (1)	
B-C	-1350 / 0	-85.5	-85.5	0.74 (1)	2.00	B-K	0 / 461	0.07 (1)	
C-D	-1350 / 0	-85.5	-85.5	0.74 (1)	2.00	K-D	-609 / 0	0.40 (1)	
D-E	-1350 / 0	-85.5	-85.5	0.74 (1)	2.00	K-F	0 / 461	0.07 (1)	
E-F	-1350 / 0	-85.5	-85.5	0.74 (1)	2.00	J-F	-1119 / 0	0.74 (1)	
F-G	-1060 / 0	-85.5	-85.5	0.71 (1)	2.00	J-G	0 / 1676	0.27 (1)	
H-G	-1562 / 0	0.0	0.0	0.73 (1)	5.27				
N-M	0 / 0	-18.5	-18.5	0.32 (4)	10.00				
M-L	0 / 0	-18.5	-18.5	0.32 (4)	10.00				
L-K	0 / 1060	-18.5	-18.5	0.44 (4)	10.00				
K-J	0 / 1060	-18.5	-18.5	0.44 (4)	10.00				
J-I	0 / 0	-18.5	-18.5	0.32 (4)	10.00				
I-H	0 / 0	-18.5	-18.5	0.32 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	21.0	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL LOAD			=	34.4 PSF

TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON
PIGGYBACK TRUSS WITH SLOPES OF 6.00/12
AND -6.00/12 AND RESPECTIVE HEEL HEIGHTS
OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD
LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.04")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (1.04")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.74/1.00 (B-D:1), BC=0.44/1.00 (K-L:4), WB=0.74/1.00 (B-L:1), SSI=0.32/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

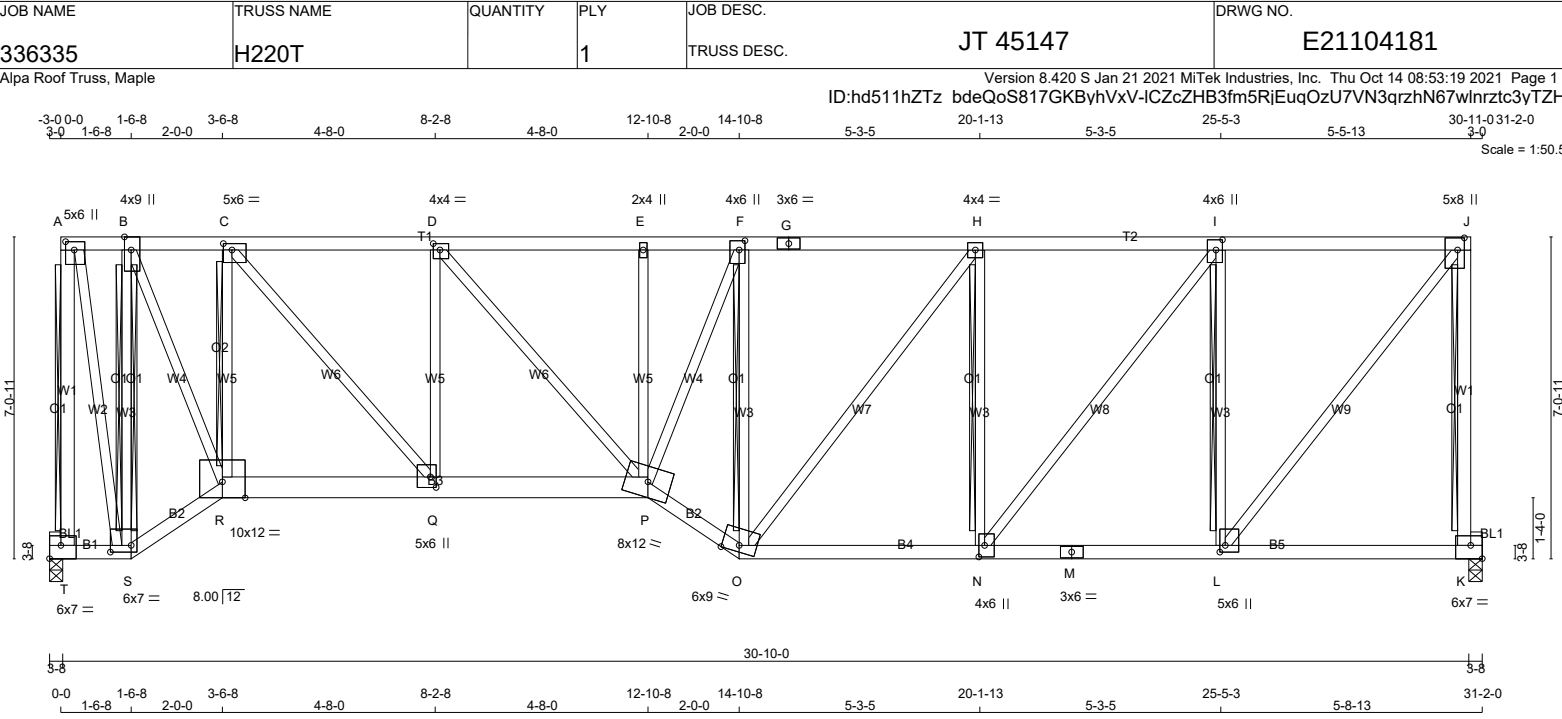
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (J) (INPUT = 0.90)
JSI METAL= 0.47 (L) (INPUT = 1.00)



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
T - A	2x4	DRY No.2	SPF
A - G	2x4	DRY 1650F 1.5E	SPF
G - J	2x4	DRY 1650F 1.5E	SPF
K - J	2x4	DRY No.2	SPF
T - S	2x4	DRY No.2	SPF
S - R	2x4	DRY No.2	SPF
R - P	2x6	DRY No.2	SPF
P - O	2x4	DRY No.2	SPF
O - M	2x4	DRY No.2	SPF
M - K	2x4	DRY No.2	SPF

BEARING BLOCKS

BL1	2 - 2x4	DRY	No.2	SPF
-----	---------	-----	------	-----

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	2.25	2.25
B	TMWW+t	MT20	4.0	9.0	Edge	1.75
C	TMWW-t	MT20	5.0	6.0	1.75	2.25
D	TMWW-t	MT20	4.0	4.0	1.75	1.75
E	TMW+w	MT20	2.0	4.0		
F	TMWW+t	MT20	4.0	6.0	2.50	1.50
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0		
I	TMWW+t	MT20	4.0	6.0	2.75	1.75
J	TMVW+p	MT20	5.0	8.0	3.25	1.75
K	BMVK1-t	MT20	6.0	7.0	3.50	Edge
L	BMWW+t	MT20	5.0	6.0	1.75	1.50
M	BS-t	MT20	3.0	6.0		
N	BMWW+t	MT20	4.0	6.0	3.00	1.50
O	BBWW-m	MT20	6.0	9.0	Edge	
P	BBWWW-m	MT20	8.0	12.0		
Q	BMWW+t	MT20	5.0	6.0	2.75	1.50
R	BBWW-t	MT20	10.0	12.0	Edge	
S	BBWW-t	MT20	6.0	7.0	1.75	5.50
T	BMVK1-t	MT20	6.0	7.0	3.50	Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
T	3150	0	3150	0
K	3158	0	3158	0

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	2245	1372 / 0	0 / 0	0 / 0	0 / 0	874 / 0	0 / 0
K	2251	1376 / 0	0 / 0	0 / 0	0 / 0	875 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, K
BEARING SIZE FACTOR = 1.15 AT JNT(S) T, K (BASED ON SUPPORT DEPTH = 1-8)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.36 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT A-T, J-K, F-O, I-L
2x4 DRY SPF No.2 I-BRACE AT B-S
2x3 DRY SPF No.2 T-BRACE AT C-R, H-N

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX LC1 (LC)	
FR-TO		FROM	TO	FR-TO			
T - A	-3122 / 0	0.0	0.0 0.88 (1)	7.81	A - S	0 / 2977	0.74 (1)
A - B	-596 / 0	-165.7	-165.7 0.18 (1)	6.25	S - B	-3125 / 0	0.98 (1)
B - C	-1769 / 0	-78.0	-78.0 0.20 (1)	5.36	B - R	0 / 3351	0.83 (1)
C - D	-3553 / 0	-78.0	-78.0 0.32 (1)	3.98	R - C	-2385 / 0	0.95 (1)
D - E	-4333 / 0	-78.0	-78.0 0.37 (1)	3.61	C - Q	0 / 2679	0.66 (1)
E - F	-4331 / 0	-78.0	-78.0 0.45 (1)	3.44	Q - D	-1306 / 0	0.63 (1)
F - G	-3587 / 0	-165.7	-165.7 0.64 (1)	3.55	D - P	0 / 1190	0.29 (1)
G - H	-3587 / 0	-165.7	-165.7 0.64 (1)	3.55	P - E	-113 / 0	0.05 (1)
H - I	-3278 / 0	-165.7	-165.7 0.80 (1)	3.36	P - F	0 / 2150	0.53 (1)
I - J	-2129 / 0	-165.7	-165.7 0.72 (1)	4.15	O - F	-2612 / 0	0.94 (1)
K - J	-3073 / 0	0.0	0.0 0.87 (1)	7.81	O - H	0 / 503	0.12 (1)
					N - H	-1260 / 0	0.87 (1)
T - S	-67 / 0	-39.3	-39.3 0.02 (4)	6.25	N - I	0 / 1868	0.46 (1)
S - R	0 / 664	-127.0	-127.0 0.21 (1)	10.00	L - I	-2477 / 0	0.89 (1)
R - Q	0 / 1815	-127.0	-127.0 0.42 (1)	10.00	L - J	0 / 3485	0.86 (1)
Q - P	0 / 3553	-127.0	-127.0 0.65 (1)	10.00			
P - O	0 / 4305	-127.0	-127.0 0.86 (1)	10.00			
O - N	0 / 3278	-39.3	-39.3 0.72 (1)	10.00			
N - M	0 / 2129	-39.3	-39.3 0.53 (1)	10.00			
M - L	0 / 2129	-39.3	-39.3 0.53 (1)	10.00			
L - K	-66 / 0	-39.3	-39.3 0.28 (4)	6.25			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 0-0
END SETBACK = 6-6-0
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 1-6-8 OF SPAN MEASURED FROM THE LEFT.

GIRDER TYPE: CStdGirder
START DISTANCE = 1-6-8
START SPAN CARRIED = 6-6-0
END DISTANCE = 14-10-8
END SPAN CARRIED = 6-6-0
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 0-0
END SETBACK = 6-6-0
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 16-3-8 OF SPAN MEASURED FROM THE RIGHT.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.02")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.25")
ALLOWABLE DEFL.(TL)= L/360 (1.02")
CALCULATED VERT. DEFL.(TL) = L/ 752 (0.49")

CSI: TC=0.88/1.00 (A-T:1) , BC=0.86/1.00 (O-P:1) , WB=0.98/1.00 (B-S:1) , SI=0.47/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

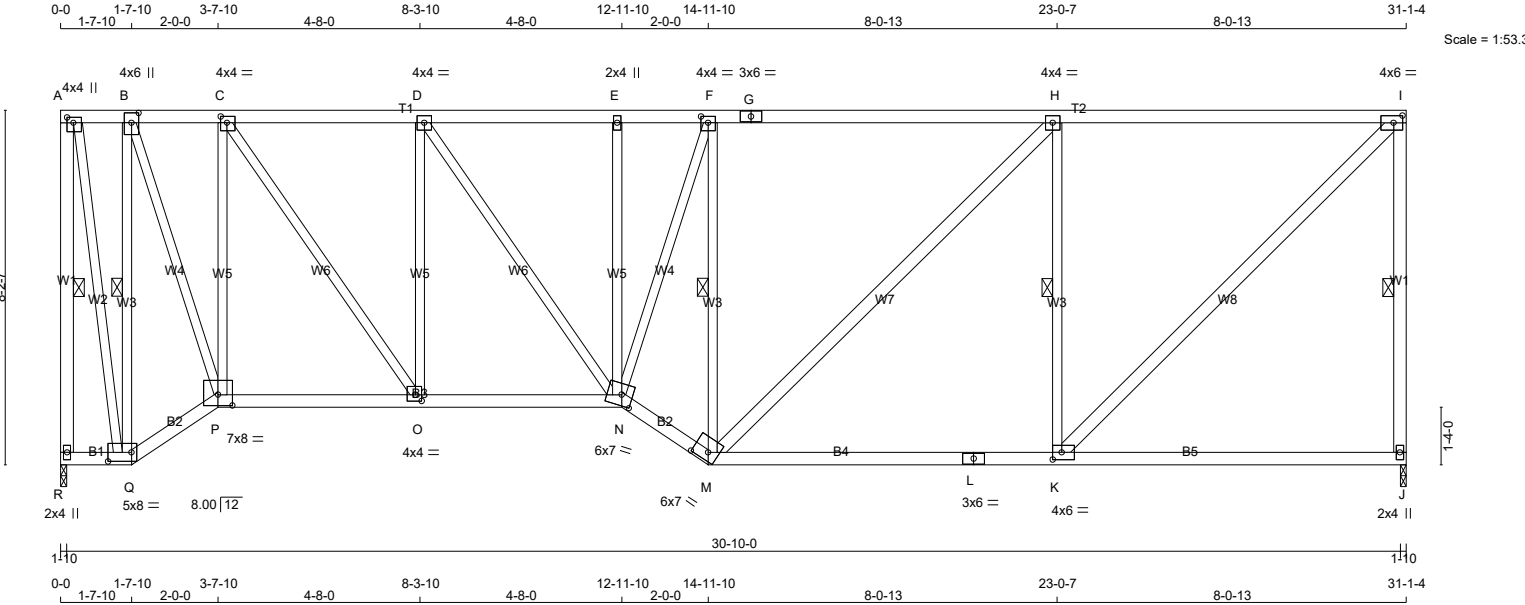
COMPANION LIVE LOAD FACTOR = 1.00

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
336335	H222T		1	TRUSS DESC. JT 45147	E21104182

Alpa Roof Truss, Maple

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Oct 14 08:53:21 2021 Page 1

ID:hd511hTZt bdeQoS817GKByhVxV-hahN zCJBNL9yY1CVOWbao88cnT a122E9S qxyTZGy



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
R - A	2x4	DRY	No.2
A - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
J - I	2x4	DRY	No.2
R - Q	2x4	DRY	No.2
Q - P	2x4	DRY	No.2
P - N	2x4	DRY	No.2
N - M	2x4	DRY	No.2
M - L	2x4	DRY	No.2
L - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
M - H	2x4	DRY	No.2
K - I	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW+p	MT20	4.0	4.0	1.50 1.75
B	TMWW+t	MT20	4.0	6.0	2.75 2.00
C	TMWW-t	MT20	4.0	4.0	1.75 1.75
D	TMWW-t	MT20	4.0	4.0	
E	TMW+w	MT20	2.0	4.0	
F	TMWW-t	MT20	4.0	4.0	1.75 2.00
G	TS-t	MT20	3.0	6.0	
H	TMWW-t	MT20	4.0	4.0	
I	TMVW-t	MT20	4.0	6.0	2.00 2.50
J	BMV1+p	MT20	2.0	4.0	
K	BMWW-t	MT20	4.0	6.0	2.00 2.50
L	BS-t	MT20	3.0	6.0	
M	BBWW-h	MT20	6.0	7.0	2.25 4.25
N	BBWW-m	MT20	6.0	7.0	3.00 3.00
O	BMWW-t	MT20	4.0	4.0	1.75 1.75
P	BBWW-l	MT20	7.0	8.0	3.00 4.00
Q	BBWW-l	MT20	5.0	8.0	2.50 6.50
R	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT		REQRD	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	BRG	IN-SX
R	1500	0	1500	0	0	1-10	1-10		
J	1500	0	1500	0	0	1-10	1-10		

UNFACTORED REACTIONS		1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
R	1070	653 / 0	0 / 0	0 / 0	0 / 0	417 / 0	0 / 0		
J	1070	653 / 0	0 / 0	0 / 0	0 / 0	417 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.78 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF A-R, I-J, B-Q, F-M, H-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	FR-TO
FR-TO		FROM	TO	FR-TO			
R-A	-1485 / 0	0.0	0.0 0.45 (1)	A-Q	0 / 1418	0.32 (1)	
A-B	-288 / 0	-78.0	-78.0 0.08 (1)	Q-B	-1533 / 0	0.65 (1)	
B-C	-744 / 0	-78.0	-78.0 0.12 (1)	B-P	0 / 1561	0.35 (1)	
C-D	-1419 / 0	-78.0	-78.0 0.27 (1)	P-C	-1253 / 0	1.00 (1)	
D-E	-1735 / 0	-78.0	-78.0 0.29 (1)	C-O	0 / 1150	0.26 (1)	
E-F	-1733 / 0	-78.0	-78.0 0.41 (1)	O-D	-848 / 0	0.68 (1)	
F-G	-1513 / 0	-78.0	-78.0 0.99 (1)	D-N	0 / 546	0.12 (1)	
G-H	-1513 / 0	-78.0	-78.0 0.99 (1)	N-E	-126 / 0	0.10 (1)	
H-I	-1221 / 0	-78.0	-78.0 0.94 (1)	E-F	0 / 773	0.17 (1)	
I-J	-1443 / 0	0.0	0.0 0.44 (1)	M-F	-1206 / 0	0.51 (1)	
R-Q	0 / 0	-18.5	-18.5 0.01 (4)	M-H	0 / 409	0.07 (1)	
Q-P	0 / 321	-18.5	-18.5 0.06 (1)	K-H	-1015 / 0	0.43 (1)	
P-O	0 / 764	-18.5	-18.5 0.18 (4)	K-I	0 / 1710	0.27 (1)	
O-N	0 / 1419	-18.5	-18.5 0.28 (1)				
N-M	0 / 1799	-18.5	-18.5 0.30 (1)				
M-L	0 / 1221	-18.5	-18.5 0.46 (4)				
L-K	0 / 1221	-18.5	-18.5 0.46 (4)				
K-J	0 / 0	-18.5	-18.5 0.35 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	34.4	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.04")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (1.04")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.25")

CSI: TC=0.99/1.00 (F-H:1) , BC=0.46/1.00 (K-M:4) , WB=1.00/1.00 (C-P:1) , SSI=0.30/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES			
PLATE GRIP(DRY)	SHEAR	SECTION	
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	650	371	1747 788 1987 1873

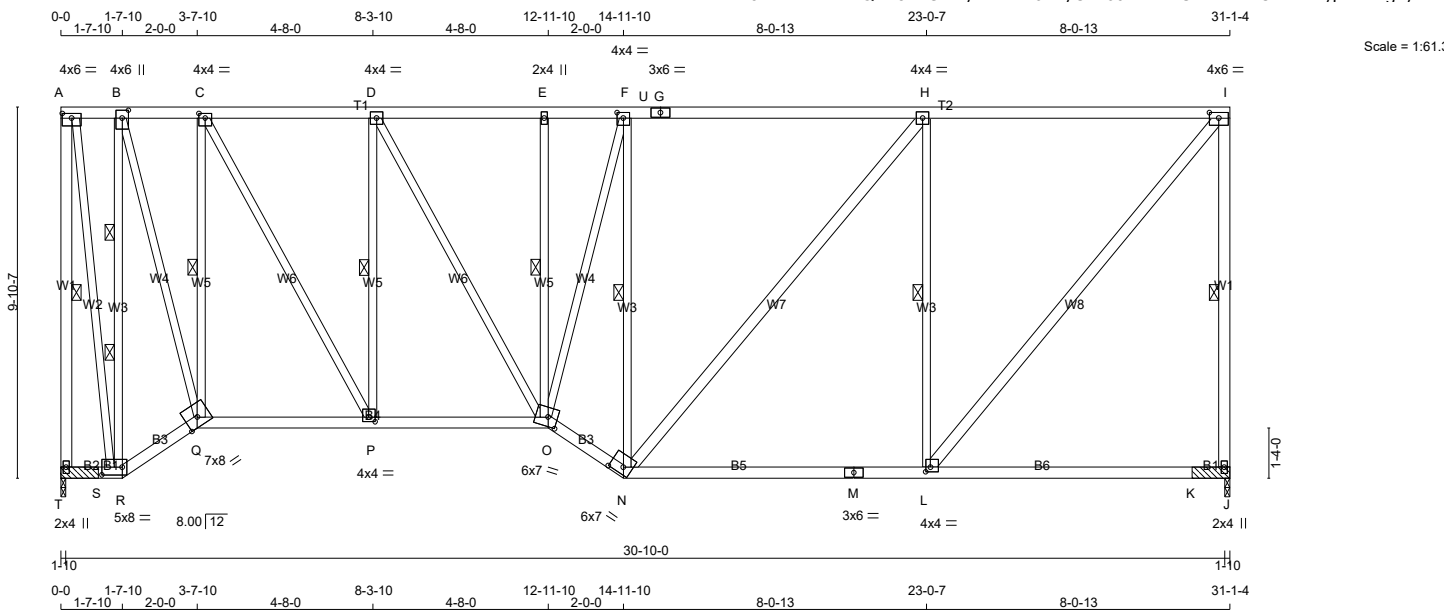
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (A) (INPUT = 0.90)
JSI METAL= 0.35 (M) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2





TOTAL WEIGHT = 3 X 201 = 603 lb

LUMBER

N. L. G. A. RULES				
CHORDS	SIZE		LUMBER	DESCR
T - A	2x4	DRY	No.2	SPF
A - G	2x4	DRY	1650F 1.5E	SPF
G - I	2x4	DRY	1650F 1.5E	SPF
J - I	2x4	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
R - Q	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
N - H	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

J	T	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0	1.50	3.00
B	TMVW+t	MT20	4.0	6.0	2.50	2.00
C	TMVW-t	MT20	4.0	4.0	1.50	2.00
D	TMVW-t	MT20	4.0	4.0		
E	TMVW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	4.0	1.75	2.00
G	TS-t	MT20	3.0	6.0		
H	TMVW-t	MT20	4.0	4.0		
I	TMVW-t	MT20	4.0	6.0	1.75	3.00
J	BMV1+p	MT20	2.0	4.0		
L	BMVW-t	MT20	4.0	4.0	1.50	1.50
M	BS-t	MT20	3.0	6.0		
N	BBVW-h	MT20	6.0	7.0	2.25	4.25
O	BBVWW-m	MT20	6.0	7.0	3.00	3.00
P	BMVW-t	MT20	4.0	4.0	1.50	2.00
Q	BBVW-h	MT20	7.0	8.0	Edge	
R	BMVW-t	MT20	5.0	8.0	2.50	6.50
T	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

LATERAL BRACE(S) SHOWN SHALL BE
2X4 SPF#2



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX
T	1617	0	1617	0	0	1-10 & BLOCK
J	1617	0	1617	0	0	1-10 & BLOCK

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1163	653/0	0/0	0/0	0/0	510/0	0/0
J	1163	653/0	0/0	0/0	0/0	510/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, J

2x4 DRY SPF No.2 BEARING BLOCK 12" LONG AT JT. T ATTACHED TO FRONT SIDE WITH 2 ROWS OF (0.122"x3") SPIRAL NAILS SPACED 3" C.C. 8 NAILS TOTAL.

2x4 DRY SPF No.2 BEARING BLOCK 12" LONG AT JT. J ATTACHED TO FRONT SIDE WITH 2 ROWS OF (0.122"x3") SPIRAL NAILS SPACED 3" C.C. 8 NAILS TOTAL.

BRACING

FOR SECTION A-I, MAX. PURLIN SPACING = 2.00 FT.

FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 0.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF A-T, I-J, C-Q, D-P, E-O, F-N, H-L.

2 LATERAL BRACE(S) AT 1/3 LENGTH OF B-R.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

C H O R D S					W E B S				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)		LC1	MAX. CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM	TO			LENGTH	FR-TO		
T-A	-1602 / 0	0.0	0.0	0.75 (1)		5.22	A-R	0 / 1510	0.34 (1)
A-B	-255 / 0	-85.5	-85.5	0.09 (1)		2.00	R-B	-1613 / 0	0.96 (1)
B-C	-639 / 0	-85.5	-85.5	0.10 (1)		2.00	B-Q	0 / 1622	0.37 (1)
C-D	-1220 / 0	-85.5	-85.5	0.22 (1)		2.00	Q-C	-1359 / 0	0.63 (1)
D-E	-1492 / 0	-85.5	-85.5	0.23 (1)		2.00	C-P	0 / 1166	0.26 (1)
E-F	-1490 / 0	-85.5	-85.5	0.35 (1)		2.00	P-D	-922 / 0	0.43 (1)
F-U	-1347 / 0	-85.5	-85.5	0.82 (1)		2.00	D-O	0 / 550	0.12 (1)
U-G	-1347 / 0	-85.5	-85.5	0.82 (1)		2.00	O-E	-137 / 0	0.06 (1)
G-H	-1347 / 0	-85.5	-85.5	0.82 (1)		2.00	O-F	0 / 625	0.14 (1)
H-I	-1087 / 0	-85.5	-85.5	0.79 (1)		2.00	N-F	-1121 / 0	0.74 (1)
J-I	-1560 / 0	0.0	0.0	0.73 (1)		5.27	N-H	0 / 404	0.06 (1)
							L-H	-1108 / 0	0.73 (1)
							L-I	0 / 1687	0.27 (1)
T-S	0 / 0	-18.5	-18.5	0.01 (4)		10.00			
S-R	0 / 0	-18.5	-18.5	0.01 (4)		10.00			
R-Q	0 / 285	-18.5	-18.5	0.06 (1)		10.00			
Q-P	0 / 656	-18.5	-18.5	0.18 (4)		10.00			
P-O	0 / 1220	-18.5	-18.5	0.25 (1)		10.00			
O-N	0 / 1605	-18.5	-18.5	0.27 (1)		10.00			
N-M	0 / 1087	-18.5	-18.5	0.46 (4)		10.00			
M-L	0 / 1087	-18.5	-18.5	0.46 (4)		10.00			
L-K	0 / 0	-18.5	-18.5	0.35 (4)		10.00			
K-J	0 / 0	-18.5	-18.5	0.35 (4)		10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL	=	21.0	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.4	PSF
TOTAL LOAD				=	34.4 PSF

TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON
PIGGYBACK TRUSS WITH SLOPES OF 6.00/12
AND -6.00/12 AND RESPECTIVE HEEL HEIGHTS
OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD
LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= $L/360$ (1.04")
 CALCULATED VERT. DEFL.(LL) = $L/999$ (0.11")
 ALLOWABLE DEFL.(TL)= $L/360$ (1.04")
 CALCULATED VERT. DEFL.(TL) = $L/999$ (0.24")

CSI: TC=0.82/1.00 (F-H:1) , BC=0.46/1.00 (L-N:4) ,
WB=0.96/1.00 (B-R:1) , SSI=0.33/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

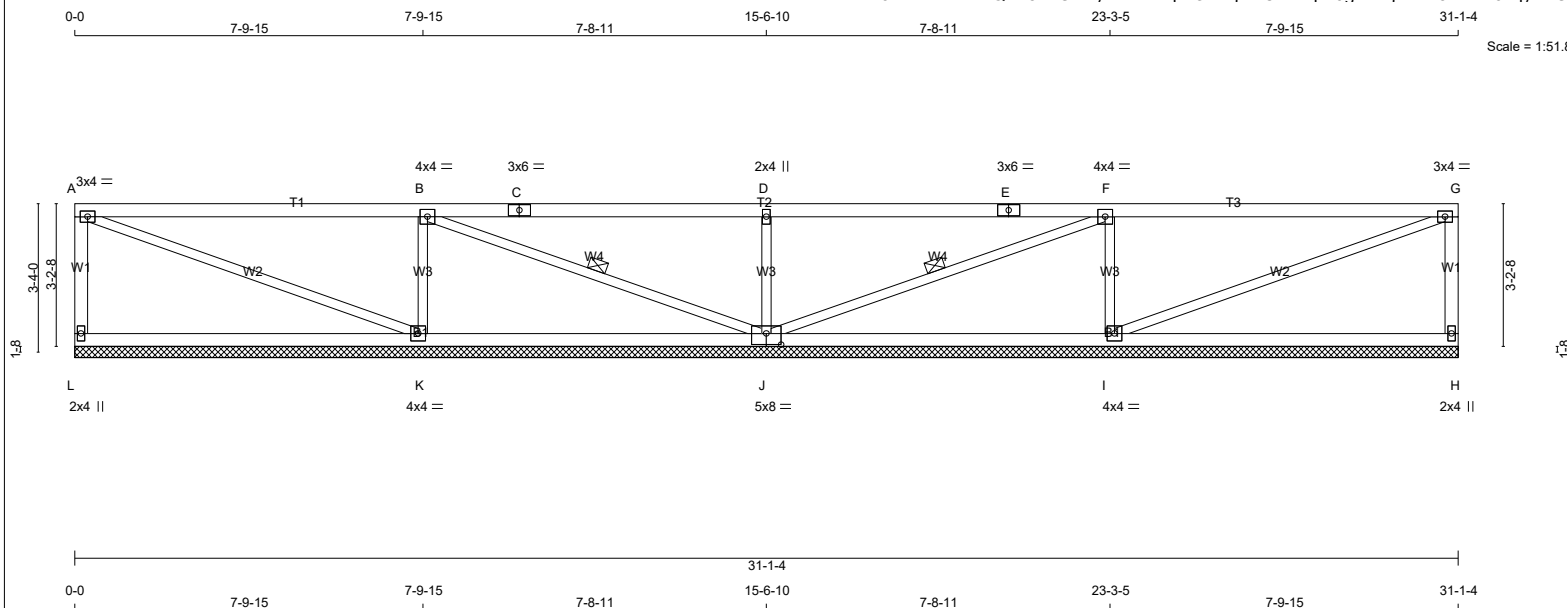
NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (L) (INPUT = 0.90)
JSI METAL= 0.38 (L) (INPUT = 1.00)



TOTAL WEIGHT = 109 lb

LUMBER				
N. L. G. A. RULES				
CHORDS		SIZE	LUMBER	DESCR
L - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	3.0	4.0		
B	TMVW-t	MT20	4.0	4.0		
C	TS-t	MT20	3.0	6.0		
D	TMV+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMVW-t	MT20	4.0	4.0		
G	TMVW-t	MT20	3.0	4.0		
H	BMV1+p	MT20	2.0	4.0		
I	BMVW1-t	MT20	4.0	4.0		
J	BSWWW1-l	MT20	5.0	8.0	3.00	4.00
K	BMVW1-t	MT20	4.0	4.0		
L	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQ'D BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT IN-SX	IN-SX
JT	296	0	296	0	0	31-1-4 (15-6-108)
K	296	0	296	0	0	31-1-4 (15-6-108)
H	854	0	854	0	0	31-1-4 (15-6-108)
J	701	0	701	0	0	31-1-4 (15-6-108)
I	854	0	854	0	0	31-1-4 (15-6-108)

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

1ST LCASE		MAX MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	211	130 / 0	0 / 0	0 / 0	0 / 0	81 / 0	0 / 0
L	211	130 / 0	0 / 0	0 / 0	0 / 0	81 / 0	0 / 0
K	610	363 / 0	0 / 0	0 / 0	0 / 0	248 / 0	0 / 0
J	497	320 / 0	0 / 0	0 / 0	0 / 0	177 / 0	0 / 0
I	610	363 / 0	0 / 0	0 / 0	0 / 0	248 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H, K, J, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF B-J, F-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX FACTORED CSI (LC)		
FR-TO		FROM TO			FR-TO				
L-A	-242 / 0	0.0	0.0	0.04 (1)	7.81	A-K	0 / 4 0.00 (1)		
A-B	-4 / 0	-78.0	-78.0	0.69 (1)	10.00	K-B	-675 / 0 0.13 (1)		
B-C	0 / 47	-78.0	-78.0	0.70 (1)	10.00	B-J	-55 / 0 0.03 (1)		
C-D	0 / 47	-78.0	-78.0	0.70 (1)	10.00	J-D	-556 / 0 0.10 (1)		
D-E	0 / 47	-78.0	-78.0	0.70 (1)	10.00	J-F	-55 / 0 0.03 (1)		
E-F	0 / 47	-78.0	-78.0	0.70 (1)	10.00	I-F	-675 / 0 0.13 (1)		
F-G	-4 / 0	-78.0	-78.0	0.69 (1)	10.00	I-G	0 / 4 0.00 (1)		
H-G	-242 / 0	0.0	0.0	0.04 (1)	7.81				
L-K	0 / 0	-18.5	-18.5	0.33 (4)	10.00				
K-J	0 / 4	-18.5	-18.5	0.33 (4)	10.00				
J-I	0 / 4	-18.5	-18.5	0.33 (4)	10.00				
I-H	0 / 0	-18.5	-18.5	0.33 (4)	10.00				

LATERAL BRACE(S) SHOWN SHALL BE
2X4 SPF#2



DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL	=	21.0	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.4	PSF
TOTAL LOAD			=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.70/1.00 (B-D:1), BC=0.33/1.00 (K-L:4),
WB=0.13/1.00 (B-K:1), SSI=0.29/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES					
PLATE	GRIP(DRY)		SHEAR		SECTION
	(PSI)		(PLI)		(PLI)
	MAX	MIN	MAX	MIN	MAX MIN
MT20	650	371	1747	788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (G) (INPUT = 0.90)
JSI METAL= 0.30 (J) (INPUT = 1.00)

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Oct 14 08:53:24 2021 Page 1



TOTAL WEIGHT = 119 lb

PLATES (table is in inches)					
JL	TYPE	PLATES	W	LEN	Y X
A	TMVW-t	MT20	3.0	4.0	
B	TMWW-t	MT20	4.0	4.0	
C	TS-t	MT20	3.0	6.0	
D	TMW+ w	MT20	2.0	4.0	
E	TS-t	MT20	3.0	6.0	
F	TMWW-t	MT20	4.0	4.0	
G	TMVW-t	MT20	3.0	4.0	
H	BMV1+p	MT20	2.0	4.0	
I	BMWW1-t	MT20	4.0	4.0	
J	BSWWWW1-t	MT20	5.0	8.0	3.00 4.00
K	BMWW1-t	MT20	4.0	4.0	
L	BMV1+p	MT20	2.0	4.0	

BEARINGS

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H, K, J, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF B-J, F-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)		
FR-TO		FROM	TO		FR-TO				
L-A	-242 / 0	0.0	0.0	0.09 (1)	7.81	A-K	0 / 3 0.00 (1)		
A-B	-3 / 0	-78.0	-78.0	0.69 (1)	10.00	K-B	-636 / 0 0.22 (1)		
B-C	0 / 96	-78.0	-78.0	0.71 (1)	10.00	B-J	-115 / 0 0.06 (1)		
C-D	0 / 96	-78.0	-78.0	0.71 (1)	10.00	J-D	-556 / 0 0.19 (1)		
D-E	0 / 96	-78.0	-78.0	0.71 (1)	10.00	J-F	-115 / 0 0.06 (1)		
E-F	0 / 96	-78.0	-78.0	0.71 (1)	10.00	I-F	-636 / 0 0.22 (1)		
F-G	-3 / 0	-78.0	-78.0	0.69 (1)	10.00	I-G	0 / 3 0.00 (1)		
H-G	-242 / 0	0.0	0.0	0.09 (1)	7.81				
L-K	0 / 0	-18.5	-18.5	0.33 (4)	10.00				
K-J	0 / 3	-18.5	-18.5	0.33 (4)	10.00				
J-I	0 / 3	-18.5	-18.5	0.33 (4)	10.00				
I-H	0 / 0	-18.5	-18.5	0.33 (4)	10.00				

LATERAL BRACE(S) SHOWN SHALL BE
2X4 SPF#2



DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	21.0	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.71/1.00 (B-D:1) , BC=0.33/1.00 (K-L:4) ,
WB=0.22/1.00 (B-K:1) , SSI=0.29/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES						
PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (A) (INPUT = 0.90)
JSI METAL= 0.33 (J) (INPUT = 1.00)

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Oct 14 08:53:26 2021 Page 1



TOTAL WEIGHT = 130 lb

PLATES (table is in inches)					
JL	TYPE	PLATES	W	LEN	Y X
A	TMVW-t	MT20	3.0	4.0	
B	TMWW-t	MT20	4.0	4.0	
C	TS-t	MT20	3.0	6.0	
D	TMW+ w	MT20	2.0	4.0	
E	TS-t	MT20	3.0	6.0	
F	TMWW-t	MT20	4.0	4.0	
G	TMVW-t	MT20	3.0	4.0	
H	BMV1+p	MT20	2.0	4.0	
I	BMWW1-t	MT20	4.0	4.0	
J	BSWWWW1-t	MT20	5.0	8.0	3.00 4.00
K	BMWW1-t	MT20	4.0	4.0	
L	BMV1+p	MT20	2.0	4.0	

	FACTORED		MAXIMUM FACTORED		INPUT	REQD
	GROSS REACTION		GROSS REACTION		BRG	BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX
JT	292	0	292	0	0	31-1-4 (15-6-108)
L	292	0	292	0	0	31-1-4 (15-6-108)
H	768	0	768	0	0	31-1-4 (15-6-108)
K	879	0	879	0	0	31-1-4 (15-6-108)
J	768	0	768	0	0	31-1-4 (15-6-108)

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	208	128 / 0	0 / 0	0 / 0	0 / 0	80 / 0	0 / 0
H	208	128 / 0	0 / 0	0 / 0	0 / 0	80 / 0	0 / 0
K	551	317 / 0	0 / 0	0 / 0	0 / 0	234 / 0	0 / 0
J	620	416 / 0	0 / 0	0 / 0	0 / 0	204 / 0	0 / 0
I	551	317 / 0	0 / 0	0 / 0	0 / 0	234 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H, K, J, I

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF A-K, B-J, F-J, G-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO			FR-TO			
L-A	-238 / 0	0.0	0.0	0.21 (1)	7.81	A-K	-4 / 0	0.00 (1)
A-B	0 / 3	-78.0	-78.0	0.69 (1)	10.00	K-B	-586 / 0	0.41 (1)
B-C	0 / 137	-78.0	-78.0	0.71 (1)	10.00	B-J	-173 / 0	0.12 (1)
C-D	0 / 137	-78.0	-78.0	0.71 (1)	10.00	J-D	-556 / 0	0.39 (1)
D-E	0 / 137	-78.0	-78.0	0.71 (1)	10.00	J-F	-173 / 0	0.12 (1)
E-F	0 / 137	-78.0	-78.0	0.71 (1)	10.00	I-F	-586 / 0	0.41 (1)
F-G	0 / 3	-78.0	-78.0	0.69 (1)	10.00	I-G	-4 / 0	0.00 (1)
H-G	-238 / 0	0.0	0.0	0.21 (1)	7.81			
L-K	0 / 0	-18.5	-18.5	0.33 (4)	10.00			
K-J	-3 / 0	-18.5	-18.5	0.33 (4)	10.00			
J-I	-3 / 0	-18.5	-18.5	0.33 (4)	10.00			
I-H	0 / 0	-18.5	-18.5	0.33 (4)	10.00			

LATERAL BRACE(S) SHOWN SHALL BE
2X4 SPF#2



SPECIFIED LOADS:

TOP	CH.	LL	=	21.0	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.4	PSF
TOTAL LOAD				=	34.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.71/1.00 (B-D:1) , BC=0.33/1.00 (K-L:4) ,
WB=0.41/1.00 (B-K:1) , SSI=0.29/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES						
PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (A) (INPUT = 0.90)
JSI METAL= 0.38 (J) (INPUT = 1.00)

Structural drawing of a roof truss system. The drawing shows a series of truss members (W1, W2, W3, W4, W5, W6, W7) and supports (A, B, C, D, E, F, G, H, I, J, K, L, M, N, O, P, Q, R, S, T, U, V, W, X, Y, Z). Dimensions are provided for various parts of the structure, including lengths (e.g., 3-0, 3-10-6, 8-1-11, 12-0-1, 6-2-13, 18-2-14, 6-2-13, 24-5-11, 6-5-5, 30-11-0, 31-2-0) and heights (e.g., 14.00, 7'-0-11, 5'-8"). The drawing is labeled with a scale of 1:51.1.

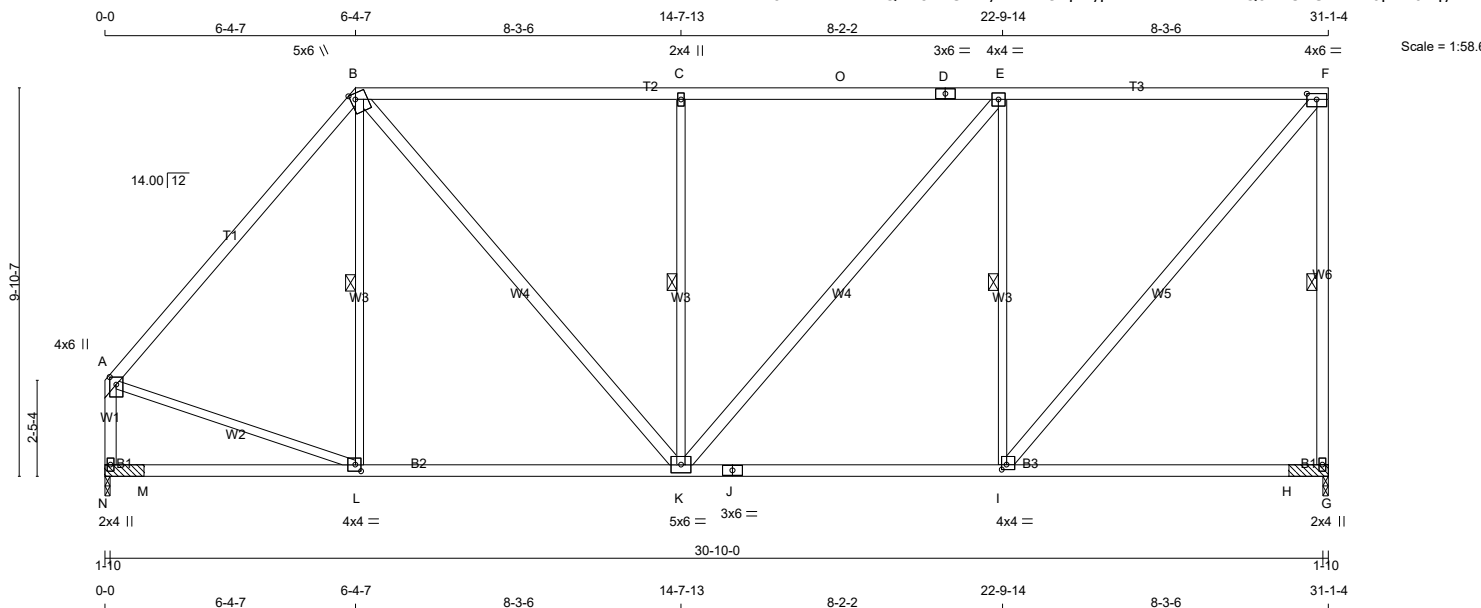
CONTINUED ON PAGE 2

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Oct 14 08:53:29 2021 Page 1



DRY: SEASONED LUMBER.

JSI GRIP= 0.84 (H) (INPUT = 0.90)
JSI METAL= 0.46 (A) (INPUT = 1.00)



TOTAL WEIGHT = 7 X 160 = 1121 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	2100F 1.8E	SPF
B - D	2x4	DRY	1650F 1.5E	SPF
D - F	2x4	DRY	1650F 1.5E	SPF
G - F	2x4	DRY	No.2	SPF
N - A	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
B - K	2x4	DRY	No.2	SPF
K - E	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
A	TMWV+p	MT20	4.0	6.0	2.25	2.00
B	TMWV+m	MT20	5.0	6.0	1.75	1.50
C	TMW+w	MT20	2.0	4.0		
D	TS-t	MT20	3.0	6.0		
E	TMWV-t	MT20	4.0	4.0		
F	TMWV-t	MT20	4.0	6.0	1.75	3.00
G	BMV1+p	MT20	2.0	4.0		
I	BMWV-t	MT20	4.0	4.0	1.50	1.50
J	BS-t	MT20	3.0	6.0		
K	BMWVWV-t	MT20	5.0	6.0		
L	BMWV-t	MT20	4.0	4.0	2.00	1.75
N	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1612	0	1612	0	0	1-10	1-10 & BLOCK
N	1574	0	1574	0	0	1-10	1-10 & BLOCK

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1159	653 / 0	0 / 0	0 / 0	0 / 0	506 / 0	0 / 0
N	1129	653 / 0	0 / 0	0 / 0	0 / 0	476 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G. N

2x4 DRY SPF No.2 BEARING BLOCK 12" LONG AT JT. G ATTACHED TO FRONT SIDE WITH 2 ROWS OF (0.122"x3") SPIRAL NAILS SPACED 3" C.C. 8 NAILS TOTAL.

2x4 DRY SPF No.2 BEARING BLOCK 12" LONG AT JT. N ATTACHED TO FRONT SIDE WITH 2 ROWS OF (0.122"x3") SPIRAL NAILS SPACED 3" C.C. 8 NAILS TOTAL.

BRACING

FOR SECTION B-F, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.10 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, B-L, C-K, E-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

C H O R D S					W E B S				
MAX. FACTORED		FACTORED			MAX. FACTORED				
MEMB.	FORCE	VERT. LOAD	LC1	MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PLF)		(LC)	UNBRAC		(LBS)	CSI (LC)	
FR-TO		FROM TO		LENGTH		FR-TO			
A-B	-1324 / 0	-78.0	-78.0	0.46 (1)	6.10	L-B	-121 / 50	0.08 (1)	
B-C	-1368 / 0	-85.5	-85.5	0.79 (1)	2.00	B-K	0 / 780	0.13 (1)	
C-O	-1369 / 0	-85.5	-85.5	0.79 (1)	2.00	K-C	-765 / 0	0.50 (1)	
O-D	-1369 / 0	-85.5	-85.5	0.79 (1)	2.00	K-E	0 / 422	0.07 (1)	
D-E	-1369 / 0	-85.5	-85.5	0.79 (1)	2.00	I-E	-1094 / 0	0.72 (1)	
E-F	-1095 / 0	-85.5	-85.5	0.76 (1)	2.00	I-F	0 / 1674	0.27 (1)	
G-F	-1551 / 0	0.0	0.0	0.73 (1)	5.29	A-L	0 / 902	0.20 (1)	
N-A	-1530 / 0	0.0	0.0	0.20 (1)	6.65				
N-M	0 / 0	-18.5	-18.5	0.24 (4)	10.00				
M-L	0 / 0	-18.5	-18.5	0.24 (4)	10.00				
L-K	0 / 858	-18.5	-18.5	0.33 (4)	10.00				
K-J	0 / 1095	-18.5	-18.5	0.43 (4)	10.00				
J-I	0 / 1095	-18.5	-18.5	0.43 (4)	10.00				
I-H	0 / 0	-18.5	-18.5	0.31 (4)	10.00				
H-G	0 / 0	-18.5	-18.5	0.31 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	21.0	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL LOAD		=	34.4	PSF

TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON
PIGGYBACK TRUSS WITH SLOPES OF 6.00/12
AND -6.00/12 AND RESPECTIVE HEEL HEIGHTS
OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD
LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.04")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (1.04")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.79/1.00 (C-E:1) , BC=0.43/1.00 (I-K:4) ,
WB=0.72/1.00 (E-I:1) , SSI=0.33/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

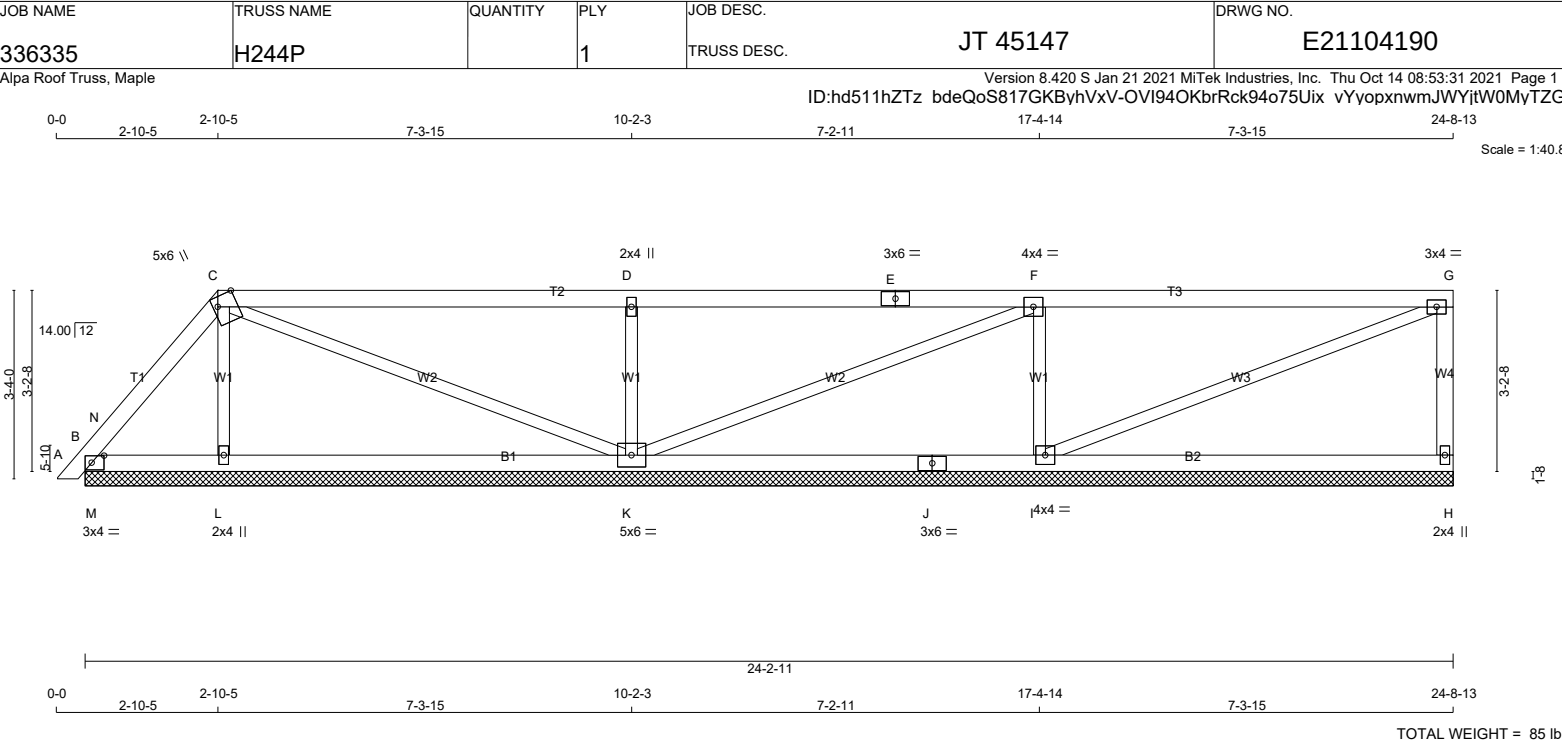
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.50 (A) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE
2X4 SPF#2





LUMBER					DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER					DESIGN CRITERIA				
N. L. G. A. RULES					BEARINGS					SPECIFIED LOADS:				
CHORDS	SIZE	LUMBER	DESCR.		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG		TOP CH.	LL			
A - C	2x4	DRY	No.2	SPF	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX			
C - E	2x4	DRY	No.2	SPF	H	288	0	288	0	24-2-11 (9-2-11)		21.0	PSF	
E - G	2x4	DRY	No.2	SPF	B	177	0	177	0	24-2-11 (9-2-11)		6.0	PSF	
H - G	2x4	DRY	No.2	SPF	L	354	0	354	0	24-2-11 (9-2-11)		0.0	PSF	
B - J	2x4	DRY	No.2	SPF	K	789	0	789	0	24-2-11 (9-2-11)		7.4	PSF	
J - H	2x4	DRY	No.2	SPF	I	759	0	759	0	24-2-11 (9-2-11)		34.4	PSF	
ALL WEBS 2x3 DRY No.2					VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH					SPACING = 24.0 IN. C/C				
DRY: SEASONED LUMBER.					UNFACTORED REACTIONS					LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0	1.50 2.50
C	TTWW+m	MT20	5.0	6.0	Edge 4.00
D	TMW+w	MT20	2.0	4.0	
E	TS-t	MT20	3.0	6.0	
F	TMWW-t	MT20	4.0	4.0	
G	TMVW-t	MT20	3.0	4.0	
H	BMV1+p	MT20	2.0	4.0	
I	BMWW1-t	MT20	4.0	4.0	
J	BS-t	MT20	3.0	6.0	
K	BMWWW1-t	MT20	5.0	6.0	
L	BMW1+w	MT20	2.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRAC LENGTH		
FR-TO		FROM	TO		FR-TO				
A-B	0 / 10	-78.0	-78.0	0.01 (1)	10.00	L-C	-220 / 0	0.04 (1)	
B-N	0 / 49	-78.0	-78.0	0.05 (1)	10.00	C-K	-62 / 0	0.08 (1)	
N-C	-87 / 0	-78.0	-78.0	0.04 (1)	6.25	K-D	-624 / 0	0.12 (1)	
C-D	0 / 16	-78.0	-78.0	0.56 (1)	10.00	K-F	-32 / 0	0.04 (1)	
D-E	0 / 16	-78.0	-78.0	0.56 (1)	10.00	I-F	-612 / 0	0.11 (1)	
E-F	0 / 16	-78.0	-78.0	0.56 (1)	10.00	I-G	0 / 14	0.00 (1)	
F-G	-13 / 0	-78.0	-78.0	0.56 (1)	6.25	M-N	-245 / 0	0.00 (1)	
H-G	-235 / 0	0.0	0.0	0.04 (1)	7.81				
B-M	0 / 49	-18.5	-18.5	0.06 (1)	10.00				
M-L	0 / 49	-18.5	-18.5	0.16 (4)	10.00				
L-K	0 / 41	-18.5	-18.5	0.19 (4)	10.00				
K-J	0 / 13	-18.5	-18.5	0.24 (4)	10.00				
J-I	0 / 13	-18.5	-18.5	0.24 (4)	10.00				
I-H	0 / 0	-18.5	-18.5	0.24 (4)	10.00				

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.56/1.00 (C-D:1) , BC=0.24/1.00 (I-K:4) , WB=0.12/1.00 (D-K:1) , SSI=0.27/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

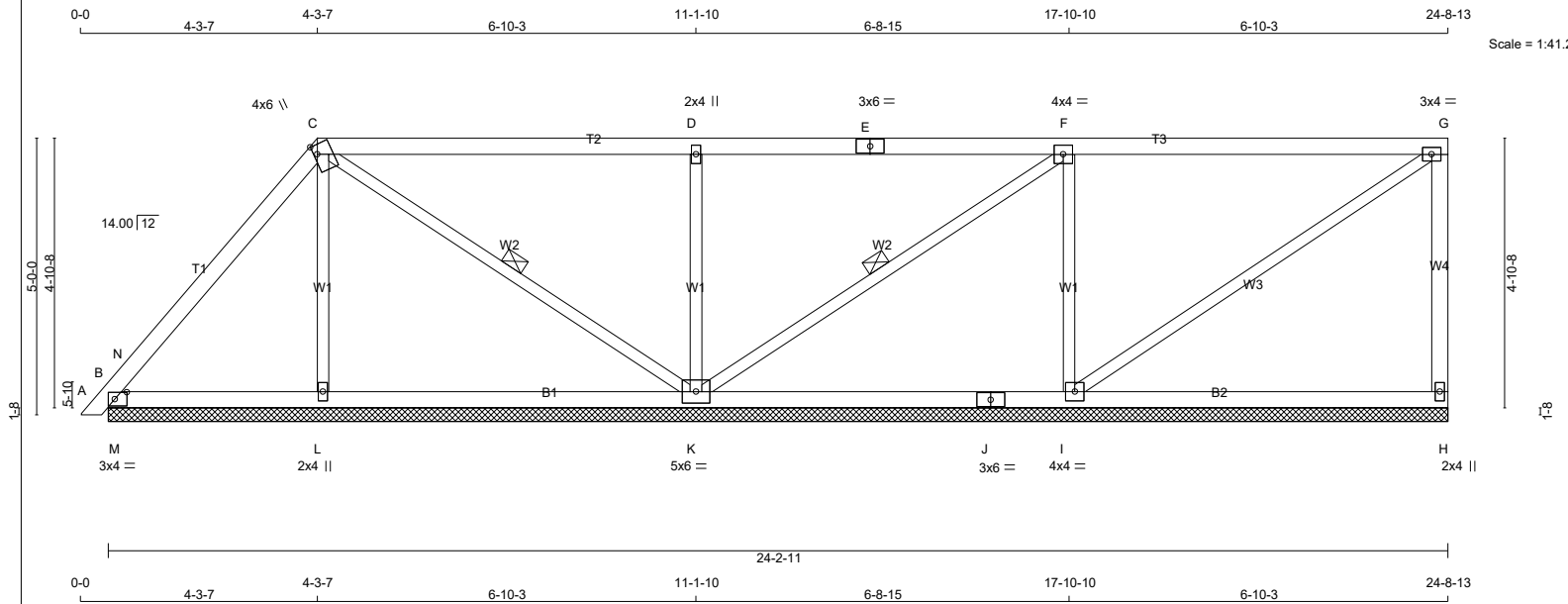
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (K) (INPUT = 0.90)
JSI METAL= 0.14 (E) (INPUT = 1.00)



TOTAL WEIGHT = 93 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	No.2	SPF
C - E	2x4	No.2	SPF
E - G	2x4	No.2	SPF
H - G	2x4	No.2	SPF
B - J	2x4	No.2	SPF
J - H	2x4	No.2	SPF

ALL WEBS 2x3 DRY
 DRY: SEASONED LUMBER.

PLATES (table is in inches)

J	T	TYPE	PLATES	W	LEN	Y	X
B	T	TM81-l	MT20	3.0	4.0	1.50	2.50
C	D	TTWW+m	MT20	4.0	6.0	2.00	0.75
B	D	TMW+w	MT20	2.0	4.0		
E	E	TS-t	MT20	3.0	6.0		
F	F	TMWW-t	MT20	4.0	4.0		
G	H	TMV-t	MT20	3.0	4.0		
H	I	BMV1+p	MT20	2.0	4.0		
I	J	BMWW1-t	MT20	4.0	4.0		
J	K	BS-t	MT20	3.0	6.0		
K	L	BMWWWW1-t	MT20	5.0	6.0		
L	M	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM GROSS REACTION		FACTORED BRG	INPUT BRG	REQ'D BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
B	280	0	280	0	0	24-2-11	(8-3-3)8
L	285	0	285	0	0	24-2-11	(8-3-3)8
K	322	0	322	0	0	24-2-11	(8-3-3)8
I	809	0	809	0	0	24-2-11	(8-3-3)8
	672	0	672	0	0	24-2-11	(8-3-3)8

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

1ST LCASE		MAX MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	200	124 / 0	0 / 0	0 / 0	0 / 0	76 / 0	0 / 0
B	200	141 / 0	0 / 0	0 / 0	0 / 0	58 / 0	0 / 0
L	235	114 / 0	0 / 0	0 / 0	0 / 0	120 / 0	0 / 0
K	573	369 / 0	0 / 0	0 / 0	0 / 0	204 / 0	0 / 0
I	480	285 / 0	0 / 0	0 / 0	0 / 0	195 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, B, L, K, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-K, F-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LOCL MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED MAX CSI (LC)	
FR-TO		FROM TO			FR-TO			
A-B	0 / 10	-78.0	-78.0	0.01 (1)	10.00	L- C	-181 / 0	0.06 (1)
B-N	0 / 167	-78.0	-78.0	0.11 (1)	10.00	C- K	-124 / 0	0.06 (1)
N- C	-129 / 0	-78.0	-78.0	0.13 (1)	6.25	K- D	-583 / 0	0.20 (1)
C- D	0 / 33	-78.0	-78.0	0.50 (1)	10.00	K- F	-68 / 0	0.03 (1)
D- E	0 / 33	-78.0	-78.0	0.50 (1)	10.00	I- F	-544 / 0	0.19 (1)
E- F	0 / 33	-78.0	-78.0	0.50 (1)	10.00	I- G	0 / 29	0.01 (1)
F- G	-24 / 0	-78.0	-78.0	0.49 (1)	6.25	M- N	-489 / 0	0.00 (1)
H- G	-230 / 0	0.0	0.0	0.09 (1)	7.81			
B- M	0 / 75	-18.5	-18.5	0.13 (1)	10.00			
M- L	0 / 75	-18.5	-18.5	0.15 (4)	10.00			
L- K	0 / 71	-18.5	-18.5	0.16 (4)	10.00			
K- J	0 / 24	-18.5	-18.5	0.21 (4)	10.00			
J- I	0 / 24	-18.5	-18.5	0.21 (4)	10.00			
I- H	0 / 0	-18.5	-18.5	0.21 (4)	10.00			

LATERAL BRACE(S) SHOWN SHALL BE
2X4 SPF#2



DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL	=	21.0	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.4	PSF
TOTAL LOAD			=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.50/1.00 (D-F:1), BC=0.21/1.00 (I-K:4),
WB=0.20/1.00 (D-K:1), SSI=0.38/1.00 (B-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

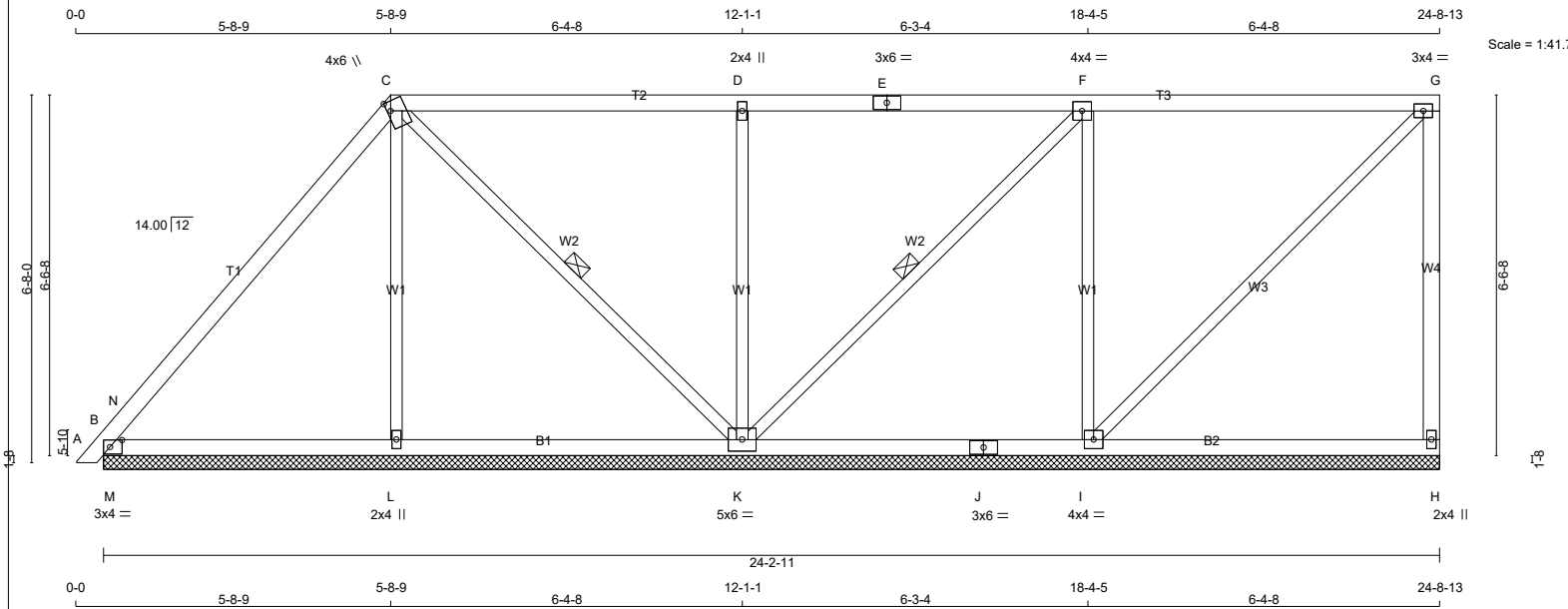
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES					
PLATE	GRIP(DRY)		SHEAR		SECTION
	(PSI)		(PLI)		(PLI)
	MAX	MIN	MAX	MIN	MAX MIN
MT20	650	371	1747	788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.23 (E) (INPUT = 1.00)



TOTAL WEIGHT = 101 lb

LUMBER			
N. L. G. A. RULES			
CHORDS		SIZE	
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
H - G	2x4	DRY	No.2
B - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS		2x3	DRY
DRY: SEASONED LUMBER.			No.2

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TM181-t	MT20	3.0	4.0	1.50	2.50
C	TTBW+m	MT20	4.0	6.0	2.00	0.75
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWV-t	MT20	4.0	4.0		
G	TMVW-t	MT20	3.0	4.0		
H	BMV1+p	MT20	2.0	4.0		
I	BMVW1-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMVWWV1-t	MT20	5.0	6.0		
L	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
BEARINGS

	FACTORED		MAXIMUM FACTORED		INPUT	REQRD
	GROSS REACTION		GROSS REACTION		BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX
B	277	0	277	0	0	24-2-11 (8-3-3)8
H	386	0	386	0	0	24-2-11 (8-3-3)8
L	287	0	287	0	0	24-2-11 (8-3-3)8
K	834	0	834	0	0	24-2-11 (8-3-3)8
I	583	0	583	0	0	24-2-11 (8-3-3)8

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	197	123 / 0	0 / 0	0 / 0	0 / 0	74 / 0	0 / 0
H	272	183 / 0	0 / 0	0 / 0	0 / 0	89 / 0	0 / 0
B	210	97 / 0	0 / 0	0 / 0	0 / 0	114 / 0	0 / 0
L	590	389 / 0	0 / 0	0 / 0	0 / 0	201 / 0	0 / 0
K	418	242 / 0	0 / 0	0 / 0	0 / 0	176 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, B, L, K, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-K, F-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (C/L)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (C/L)
FR-TO		FROM TO			FR-TO		
A-B	0 / 10	-78.0	-78.0 0.01 (1)	10.00	L-C	-123 / 0	0.09 (1)
B-N	0 / 410	-78.0	-78.0 0.23 (1)	10.00	C-K	-183 / 0	0.10 (1)
N-C	-176 / 0	-78.0	-78.0 0.26 (1)	6.25	K-D	-542 / 0	0.38 (1)
C-D	0 / 31	-78.0	-78.0 0.43 (1)	10.00	K-F	-89 / 0	0.05 (1)
D-E	0 / 31	-78.0	-78.0 0.43 (1)	10.00	I-F	-479 / 0	0.34 (1)
E-F	0 / 31	-78.0	-78.0 0.43 (1)	10.00	I-G	0 / 45	0.01 (1)
F-G	-32 / 0	-78.0	-78.0 0.43 (1)	6.25	M-N	-879 / 0	0.00 (1)
H-G	-231 / 0	0.0	0.0 0.20 (1)	7.81			
B- M	0 / 102	-18.5	-18.5 0.22 (1)	10.00			
M- L	0 / 102	-18.5	-18.5 0.22 (1)	10.00			
L- K	0 / 100	-18.5	-18.5 0.17 (4)	10.00			
K- J	0 / 32	-18.5	-18.5 0.19 (4)	10.00			
J- I	0 / 32	-18.5	-18.5 0.19 (4)	10.00			
I- H	0 / 0	-18.5	-18.5 0.19 (4)	10.00			

LATERAL BRACE(S) SHOWN SHALL BE
2X4 SPF#2



DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	21.0	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.43/1.00 (D-F:1), BC=0.22/1.00 (B-M:1),
WB=0.38/1.00 (D-K:1), SSI=0.69/1.00 (B-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

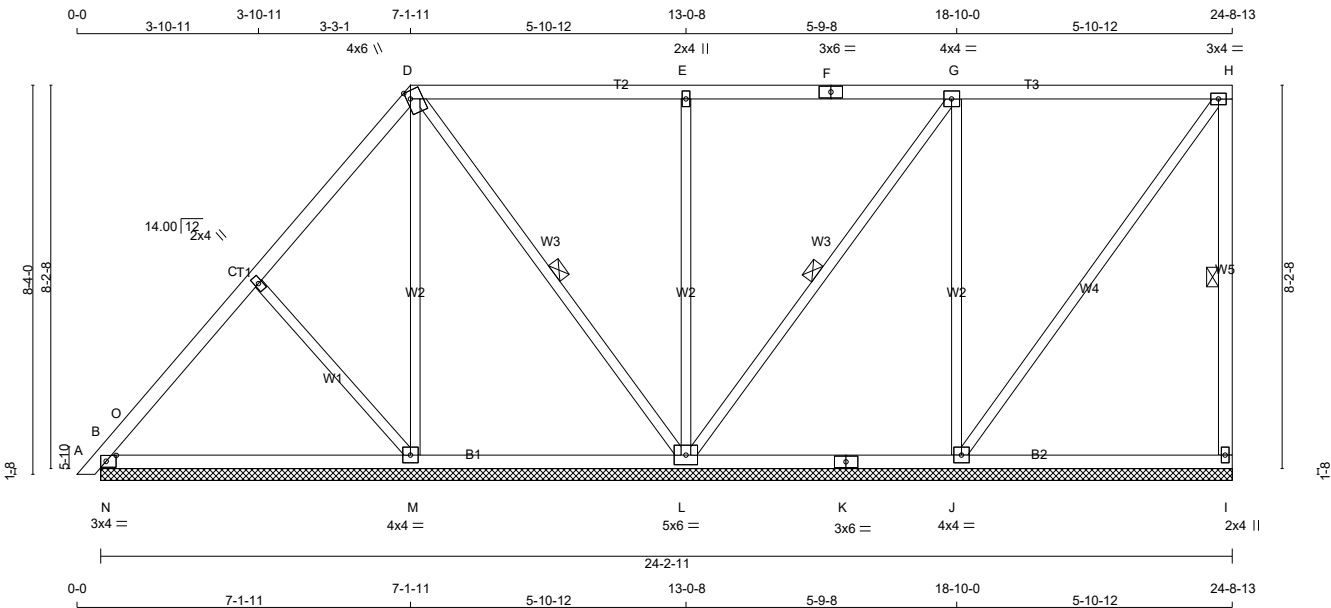
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES						
PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.17 (E) (INPUT = 1.00)



LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER								DESIGN CRITERIA			
N. L. G. A. RULES				BEARINGS								SPECIFIED LOADS:			
CHORDS	SIZE	LUMBER	DESCR.	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG	TOP CH.	LL	PSF			
				VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX		DL	PSF			
A - D	2x4	DRY	No.2												
D - F	2x4	DRY	No.2												
F - H	2x4	DRY	No.2												
I - H	2x4	DRY	No.2												
B - K	2x4	DRY	No.2												
K - I	2x4	DRY	No.2												
ALL WEBS 2x3 DRY: SEASONED LUMBER.															
				JT											
				I	228	0	228	0	0	24-2-11 (8-3-3)8					
				B	340	0	340	0	0	24-2-11 (8-3-3)8					
				M	460	0	460	0	0	24-2-11 (8-3-3)8					
				L	842	0	842	0	0	24-2-11 (8-3-3)8					
				J	498	0	498	0	0	24-2-11 (8-3-3)8					
BOT CH. LL = 0.0 PSF															
DL = 7.4 PSF															
TOTAL LOAD = 34.4 PSF															
SPACING = 24.0 IN. C/C															

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

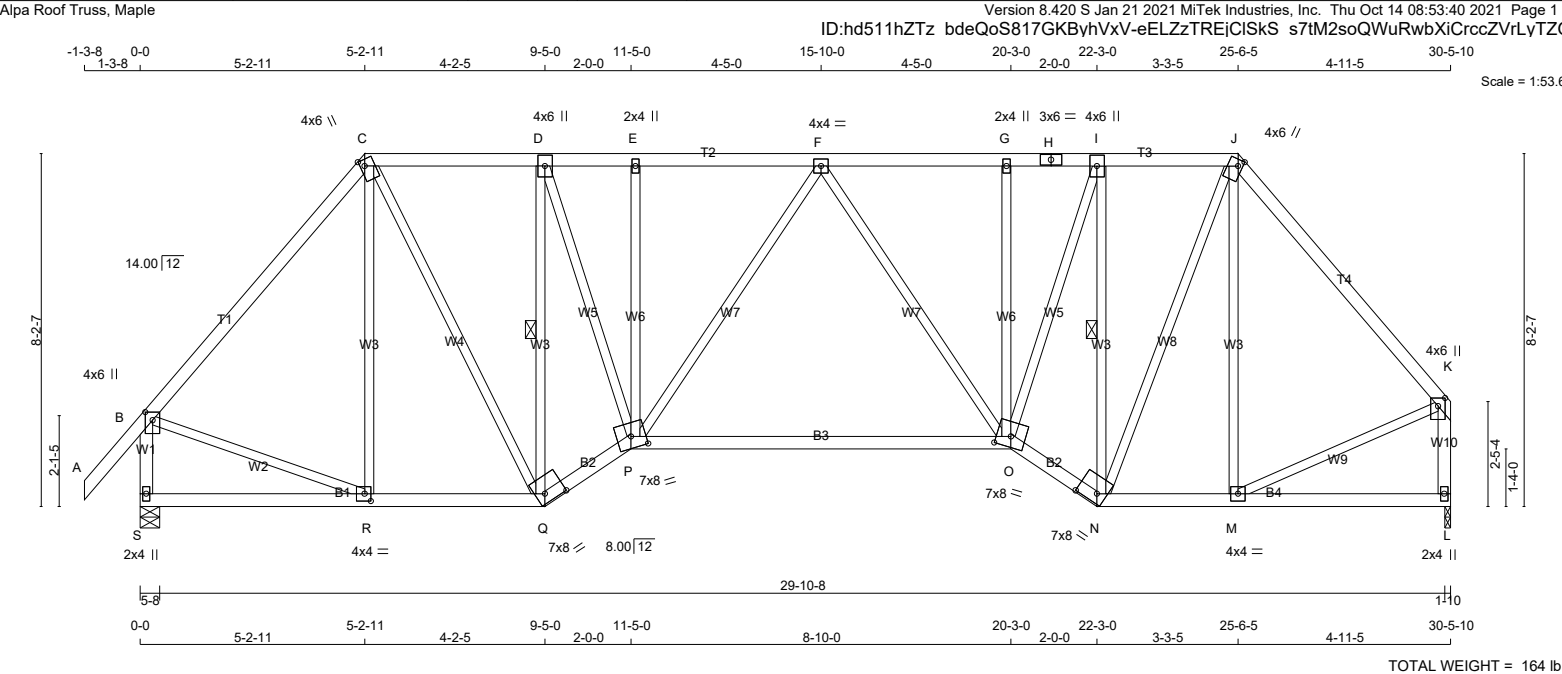


Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Oct 14 08:53:38 2021 Page 1



CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
336335	H262T		1	TRUSS DESC. JT 45147	E21104195



LUMBER				DESCR.			
N. L. G. A. RULES							
CHORDS	SIZE	LUMBER		SPF			
A - C	2x4	DRY	No.2	SPF			
C - H	2x4	DRY	No.2	SPF			
H - J	2x4	DRY	No.2	SPF			
J - K	2x4	DRY	No.2	SPF			
S - B	2x4	DRY	No.2	SPF			
L - K	2x4	DRY	No.2	SPF			
S - Q	2x4	DRY	No.2	SPF			
Q - P	2x4	DRY	No.2	SPF			
P - O	2x4	DRY	No.2	SPF			
O - N	2x4	DRY	No.2	SPF			
N - L	2x4	DRY	No.2	SPF			
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF			

DRY: SEASONED LUMBER.

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.25	2.00
C	TTWW+m	MT20	4.0	6.0	1.75	1.25
D	TMWW+t	MT20	4.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMW+w	MT20	2.0	4.0		
H	TS-t	MT20	3.0	6.0		
I	TMWW+t	MT20	4.0	6.0		
J	TTWW+m	MT20	4.0	6.0	1.75	1.25
K	TMVW+p	MT20	4.0	6.0	2.25	2.00
L	BMV1+p	MT20	2.0	4.0		
M	BMWW-t	MT20	4.0	4.0		
N	BBWW-h	MT20	7.0	8.0	2.50	5.50
O	BBWWW-m	MT20	7.0	8.0	3.00	4.00
P	BBWWW-m	MT20	7.0	8.0	3.25	4.00
Q	BBWW-h	MT20	7.0	8.0	2.50	5.50
R	BMWW-t	MT20	4.0	4.0	2.00	1.75
S	BMV1+p	MT20	2.0	4.0		

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	S	L	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
			VERT	HORZ	DOWN	HORZ		
			1579	0	1579	0	5-8	1-11
			1470	0	1470	0	1-10	1-9

UNFACTORED REACTIONS							
1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1124	698 / 0	0 / 0	0 / 0	0 / 0	425 / 0	0 / 0
L	1048	640 / 0	0 / 0	0 / 0	0 / 0	408 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.95 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-Q, I-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 42	-78.0 -78.0	0.11 (1)	10.00	R-C	-160 / 17	0.21 (1)
B-C	-1261 / 0	-78.0 -78.0	0.48 (1)	5.19	C-Q	0 / 872	0.20 (1)
C-D	-1226 / 0	-78.0 -78.0	0.18 (1)	5.58	Q-D	-1528 / 0	0.65 (1)
D-E	-1604 / 0	-78.0 -78.0	0.17 (1)	5.05	D-P	0 / 1304	0.29 (1)
E-F	-1607 / 0	-78.0 -78.0	0.25 (1)	4.95	P-E	-201 / 0	0.16 (1)
F-G	-1531 / 0	-78.0 -78.0	0.25 (1)	5.05	P-F	-164 / 0	0.21 (1)
G-H	-1527 / 0	-78.0 -78.0	0.10 (1)	5.24	F-O	-300 / 0	0.39 (1)
H-I	-1527 / 0	-78.0 -78.0	0.10 (1)	5.24	O-G	-233 / 0	0.19 (1)
I-J	-1124 / 0	-78.0 -78.0	0.12 (1)	5.85	O-I	0 / 1384	0.31 (1)
J-K	-1207 / 0	-78.0 -78.0	0.41 (1)	5.35	N-I	-1527 / 0	0.65 (1)
S-B	-1540 / 0	0.0 0.0	0.18 (1)	6.63	N-J	0 / 896	0.20 (1)
L-K	-1433 / 0	0.0 0.0	0.19 (1)	6.82	M-J	-223 / 0	0.29 (1)
					B-R	0 / 860	0.19 (1)
					M-K	0 / 846	0.19 (1)
S-R	0 / 0	-18.5 -18.5	0.12 (4)	10.00			
R-Q	0 / 815	-18.5 -18.5	0.20 (1)	10.00			
Q-P	0 / 1449	-18.5 -18.5	0.25 (1)	10.00			
P-O	0 / 1699	-18.5 -18.5	0.58 (4)	10.00			
O-N	0 / 1327	-18.5 -18.5	0.23 (1)	10.00			
N-M	0 / 780	-18.5 -18.5	0.19 (1)	10.00			
M-L	0 / 0	-18.5 -18.5	0.11 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:			
TOP CH.	LL	= 21.0	PSF
	DL	= 6.0	PSF
BOT CH.	LL	= 0.0	PSF
	DL	= 7.4	PSF
TOTAL LOAD = 34.4 PSF			

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.02")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (1.02")
CALCULATED VERT. DEFL.(TL)= L/ 815 (0.45")

CSI: TC=0.48/1.00 (B-C:1) , BC=0.58/1.00 (O-P:4) , WB=0.65/1.00 (D-Q:1) , SSI=0.15/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES			
PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

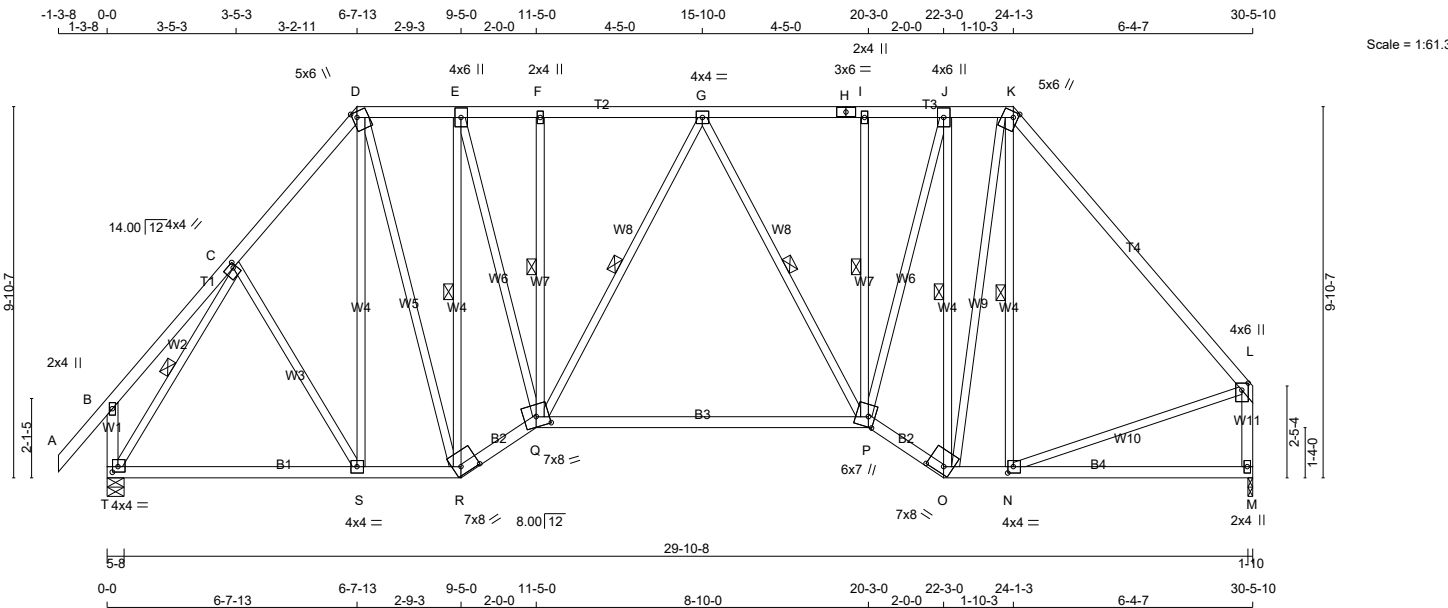
JSI GRIP= 0.89 (P) (INPUT = 0.90)
JSI METAL= 0.50 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
336335	H263T		1	TRUSS DESC. JT 45147	E21104196

Alpa Roof Truss, Maple

ID:hd511hZTz bdeQoS817GkByhVxV-adSJO9TUfQ?A m8EEIOWxDWtFdZ?YQ84w2bvEvTZGd

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Oct 14 08:53:42 2021 Page 1



TOTAL WEIGHT = 2 X 188 = 376 lb [M]

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
D - H	2x4	DRY No.2	SPF
H - K	2x4	DRY No.2	SPF
K - L	2x4	DRY 2100F 1.8E	SPF
T - B	2x4	DRY No.2	SPF
M - L	2x4	DRY No.2	SPF
T - R	2x4	DRY No.2	SPF
R - Q	2x4	DRY No.2	SPF
Q - P	2x4	DRY No.2	SPF
P - O	2x4	DRY No.2	SPF
O - M	2x4	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	2.0	4.0	
C	TMWW-t	MT20	4.0	4.0	1.50 1.00
D	TTWW+m	MT20	5.0	6.0	1.75 1.50
E	TMWW+t	MT20	4.0	6.0	
F	TMW+w	MT20	2.0	4.0	
G	TMWW-t	MT20	4.0	4.0	
H	TS-t	MT20	3.0	6.0	
I	TMW+w	MT20	2.0	4.0	
J	TMWW+t	MT20	4.0	6.0	
K	TTWW+m	MT20	5.0	6.0	1.75 1.50
L	TMVW+p	MT20	4.0	6.0	2.25 2.00
M	BMV1+p	MT20	2.0	4.0	
N	BMWW-t	MT20	4.0	4.0	2.00 1.75
O	BBWW-h	MT20	7.0	8.0	2.25 5.25
P	BBWWW+m	MT20	6.0	7.0	3.25 2.00
Q	BBWWW-m	MT20	7.0	8.0	3.25 4.00
R	BBWW-h	MT20	7.0	8.0	2.50 5.50
S	BMWW-t	MT20	4.0	4.0	
T	BMVW1-t	MT20	4.0	4.0	1.75 1.75

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX		REQRD BRG IN-SX	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
JT	1470	0	1470	0	0	1-10	1-9	
M	1470	0	1470	0	0	5-8	1-11	

UNFACTORED REACTIONS							
	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	1048	640 / 0	0 / 0	0 / 0	0 / 0	408 / 0	0 / 0
M	1124	698 / 0	0 / 0	0 / 0	0 / 0	425 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, T

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.42 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-R, F-Q, G-Q, G-P, I-P, J-O, K-N, C-T.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)		MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED MAX. UNBRACED LENGTH (FT)	
		FROM	TO			FR-TO	
A-B	0 / 42	-78.0	-78.0 0.11 (1)	10.00	C-S	0 / 61	0.02 (4)
B-C	0 / 22	-78.0	-78.0 0.13 (1)	10.00	S-D	0 / 93	0.03 (4)
C-D	-1271 / 0	-78.0	-78.0 0.16 (1)	5.55	D-R	0 / 694	0.16 (1)
D-E	-1003 / 0	-78.0	-78.0 0.09 (1)	6.14	R-E	-1308 / 0	0.86 (1)
E-F	-1280 / 0	-78.0	-78.0 0.09 (1)	5.61	E-Q	0 / 1174	0.26 (1)
F-G	-1283 / 0	-78.0	-78.0 0.23 (1)	5.42	Q-F	-246 / 0	0.11 (1)
G-H	-1222 / 0	-78.0	-78.0 0.23 (1)	5.52	Q-G	-150 / 0	0.09 (1)
H-I	-1222 / 0	-78.0	-78.0 0.23 (1)	5.52	G-P	-279 / 0	0.17 (1)
I-J	-1219 / 0	-78.0	-78.0 0.10 (1)	5.70	P-I	-276 / 0	0.13 (1)
J-K	-920 / 0	-78.0	-78.0 0.07 (1)	6.25	P-J	0 / 1265	0.28 (1)
K-L	-1214 / 0	-78.0	-78.0 0.45 (1)	6.25	O-J	-1300 / 0	0.85 (1)
T-B	-208 / 0	0.0	0.0 0.02 (1)	7.81	O-K	0 / 700	0.16 (1)
M-L	-1423 / 0	0.0	0.0 0.19 (1)	6.84	N-K	-118 / 47	0.08 (1)
					T-C	-1531 / 0	0.44 (1)
					N-L	0 / 827	0.19 (1)
T-S	0 / 777	-18.5	-18.5 0.26 (4)	10.00			
S-R	0 / 810	-18.5	-18.5 0.26 (4)	10.00			
R-Q	0 / 1189	-18.5	-18.5 0.20 (1)	10.00			
Q-P	0 / 1354	-18.5	-18.5 0.55 (4)	10.00			
P-O	0 / 1088	-18.5	-18.5 0.19 (1)	10.00			
O-N	0 / 786	-18.5	-18.5 0.26 (4)	10.00			
N-M	0 / 0	-18.5	-18.5 0.18 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:			
TOP CH.	LL	=	21.0 PSF
	DL	=	6.0 PSF
BOT CH.	LL	=	0.0 PSF
	DL	=	7.4 PSF
TOTAL LOAD = 34.4 PSF			

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.02")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (1.02")
CALCULATED VERT. DEFL.(TL)= L/ 853 (0.43")

CSI: TC=0.45/1.00 (K-L:1) , BC=0.55/1.00 (P-Q:4) , WB=0.86/1.00 (E-R:1) , SSI=0.15/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

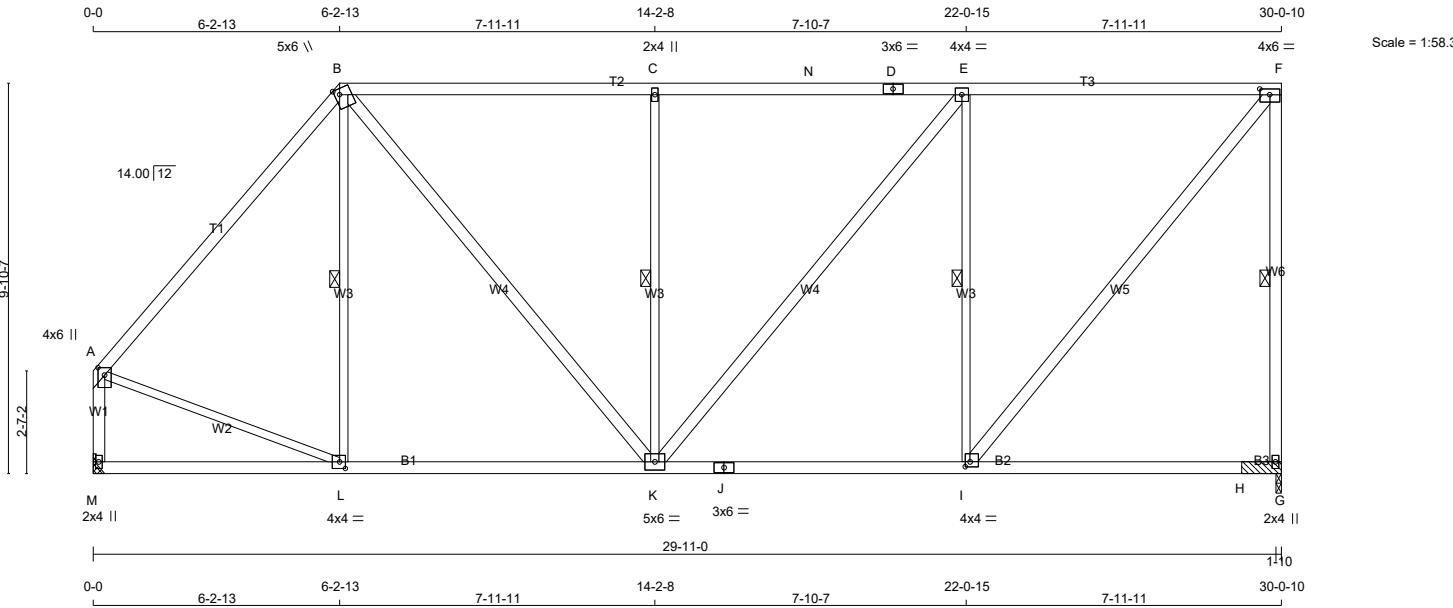
JSI GRIP= 0.87 (Q) (INPUT = 0.90)
JSI METAL= 0.51 (C) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
336335	H263A		1	TRUSS DESC. JT 45147	E21104197

Alpa Roof Truss, Maple

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Oct 14 08:52:43 2021 Page 1

ID:hd511hZTz bdeQoS817GKByhVxV-oGO??? IU8iZYPi7vHPrg2nZidSW9gwOSVFF L8yTZHY



TOTAL WEIGHT = 3 X 156 = 468 lb

[M]

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - B	2x4	DRY	2100F 1.8E	SPF
B - D	2x4	DRY	1650F 1.5E	SPF
D - F	2x4	DRY	1650F 1.5E	SPF
G - F	2x4	DRY	No.2	SPF
M - A	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
B - K	2x4	DRY	No.2	SPF
K - E	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW+p	MT20	4.0	6.0	2.25 2.00
B	TTWW+m	MT20	5.0	6.0	1.75 1.50
C	TMW+w	MT20	2.0	4.0	
D	TS-t	MT20	3.0	6.0	
E	TMWW-t	MT20	4.0	4.0	
F	TMVW-t	MT20	4.0	6.0	1.75 3.00
G	BMV1+p	MT20	2.0	4.0	
I	BMWW-t	MT20	4.0	4.0	1.50 1.50
J	BS-t	MT20	3.0	6.0	
K	BMWWW-t	MT20	5.0	6.0	
L	BMWW-t	MT20	4.0	4.0	2.00 1.75
M	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ		DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
G	1557	0		1557	0	0	1-10	1-10 & BLOCK	
M	1520	0		1520	0	0	MECHANICAL		

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT M. MINIMUM BEARING LENGTH AT JOINT M = 1-10.

UNFACTORED REACTIONS

		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1120	631 / 0	0 / 0	0 / 0	0 / 0	489 / 0	0 / 0
M	1090	631 / 0	0 / 0	0 / 0	0 / 0	459 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

2x4 DRY SPF No.2 BEARING BLOCK 12" LONG AT JT. G ATTACHED TO FRONT SIDE WITH 2 ROWS OF (0.122"x3") SPIRAL NAILS SPACED 3" C.C. 8 NAILS TOTAL.

BRACING

FOR SECTION B-F, MAX. PURLIN SPACING = 2.00 FT.

FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, B-L, C-K, E-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)	MEMB.	FORCE (LBS)	FACTORED MAX CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	-1248 / 0	-78.0 -78.0	0.43 (1)	L-B	-136 / 41	0.09 (1)	
B-C	-1277 / 0	-85.5 -85.5	0.71 (1)	B-K	0 / 732	0.12 (1)	
C-N	-1277 / 0	-85.5 -85.5	0.71 (1)	K-C	-737 / 0	0.48 (1)	
N-D	-1277 / 0	-85.5 -85.5	0.71 (1)	K-E	0 / 406	0.07 (1)	
D-E	-1277 / 0	-85.5 -85.5	0.71 (1)	I-E	-1058 / 0	0.70 (1)	
E-F	-1020 / 0	-85.5 -85.5	0.69 (1)	I-F	0 / 1594	0.26 (1)	
G-F	-1499 / 0	0.0 0.0	0.70 (1)	A-L	0 / 858	0.19 (1)	
M-A	-1477 / 0	0.0 0.0	0.21 (1)				
M-L	0 / 0	-18.5 -18.5	0.22 (4)				
L-K	0 / 808	-18.5 -18.5	0.31 (4)				
K-J	0 / 1020	-18.5 -18.5	0.40 (4)				
J-I	0 / 1020	-18.5 -18.5	0.40 (4)				
I-H	0 / 0	-18.5 -18.5	0.28 (4)				
H-G	0 / 0	-18.5 -18.5	0.28 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 21.0 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE HEEL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")

CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")

ALLOWABLE DEFL.(TL)= L/360 (1.00")

CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.71/1.00 (C-E:1) , BC=0.40/1.00 (I-K:4) , WB=0.70/1.00 (E-I:1) , SSI=0.32/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (I) (INPUT = 0.90)

JSI METAL= 0.48 (A) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

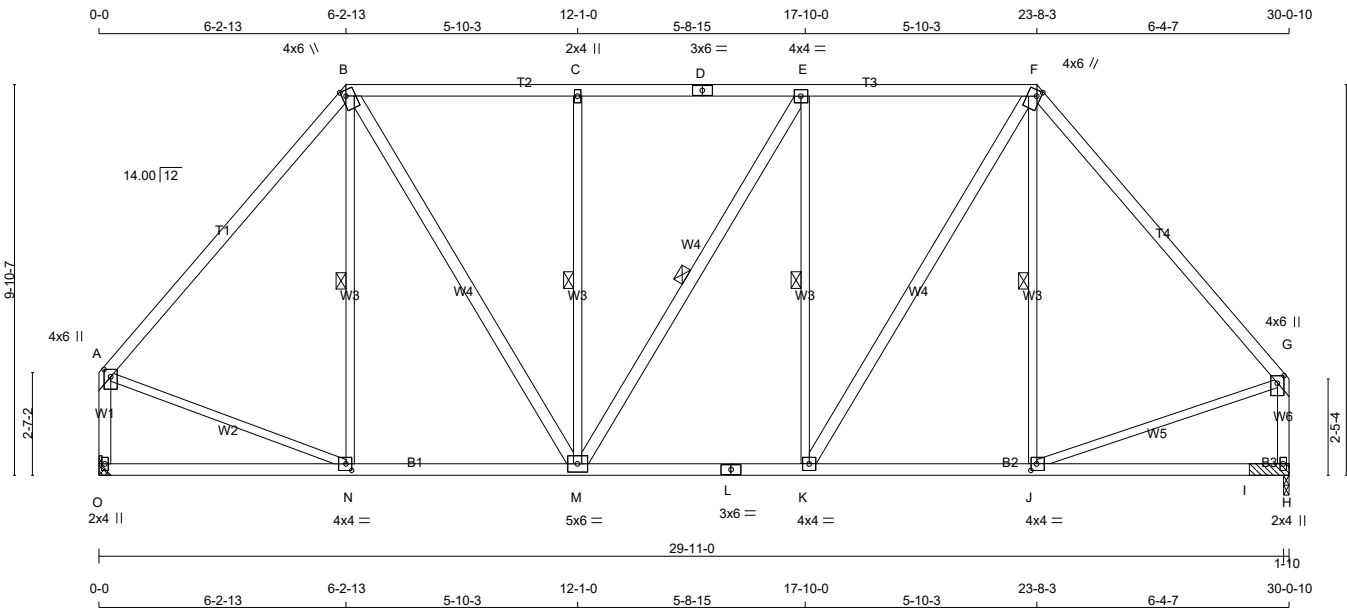


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
336335	H263S		1	TRUSS DESC. JT 45147	E21104198

Alpa Roof Truss, Maple

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Oct 14 08:52:45 2021 Page 1

ID:hd511hZTz bdeQoS817GKBvhVxV-lfWmQfnkgJpGe?HHOqt87Ce6BGEq8vGlzZk4P1yTZHW



TOTAL WEIGHT = 3 X 161 = 482 lb [M]

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY 2100F 1.8E	SPF
B - D	2x4	DRY No.2	SPF
D - F	2x4	DRY No.2	SPF
F - G	2x4	DRY 2100F 1.8E	SPF
O - A	2x4	DRY No.2	SPF
H - G	2x4	DRY No.2	SPF
O - L	2x4	DRY No.2	SPF
L - H	2x4	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF
B - M	2x4	DRY No.2	SPF
M - E	2x4	DRY No.2	SPF
K - F	2x4	DRY No.2	SPF
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW+p	MT20	4.0	6.0	2.25 2.00
B	TTWW+m	MT20	4.0	6.0	1.75 1.25
C	TMW+w	MT20	2.0	4.0	
D	TS-t	MT20	3.0	6.0	
E	TMWW-t	MT20	4.0	4.0	
F	TTWW+m	MT20	4.0	6.0	1.75 1.25
G	TMVW+p	MT20	4.0	6.0	2.25 2.00
H	BMV1+p	MT20	2.0	4.0	
J	BMWW-t	MT20	4.0	4.0	2.00 1.75
K	BMWW-t	MT20	4.0	4.0	
L	BS-t	MT20	3.0	6.0	
M	BMWWW-t	MT20	5.0	6.0	
N	BMWW-t	MT20	4.0	4.0	2.00 1.75
O	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
O	1515	0	1515	0	0	MECHANICAL			
H	1515	0	1515	0	0	1-10	1-10	& BLOCK	
A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT O. MINIMUM BEARING LENGTH AT JOINT O = 1-10.									

UNFACTORED REACTIONS							
JT	1ST LCASE	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
O	1086	631 / 0	0 / 0	0 / 0	0 / 0	0 / 0	455 / 0
H	1086	631 / 0	0 / 0	0 / 0	0 / 0	0 / 0	455 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

2x4 DRY SPF No.2 BEARING BLOCK 12" LONG AT JT. H ATTACHED TO FRONT SIDE WITH 2 ROWS OF (0.122"x3") SPIRAL NAILS SPACED 3" C.C. 8 NAILS TOTAL.

BRACING
FOR SECTION B-F, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.21 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF B-N, C-M, E-M, E-K, F-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)	FACTORED MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	-1239 / 0	-78.0 -78.0	0.43 (1)	6.25	N-B	-152 / 21	0.10 (1)
B-C	-1196 / 0	-85.5 -85.5	0.44 (1)	2.00	B-M	0 / 753	0.12 (1)
C-D	-1196 / 0	-85.5 -85.5	0.44 (1)	2.00	M-C	-534 / 0	0.35 (1)
D-E	-1196 / 0	-85.5 -85.5	0.44 (1)	2.00	M-E	-10 / 0	0.01 (1)
E-F	-1201 / 0	-85.5 -85.5	0.44 (1)	2.00	K-E	-528 / 0	0.35 (1)
F-G	-1258 / 0	-78.0 -78.0	0.45 (1)	6.21	K-F	0 / 739	0.12 (1)
O-A	-1468 / 0	0.0 0.0	0.20 (1)	6.76	J-F	-131 / 32	0.09 (1)
H-G	-1466 / 0	0.0 0.0	0.19 (1)	6.76	A-N	0 / 852	0.19 (1)
					J-G	0 / 857	0.19 (1)
O-N	0 / 0	-18.5 -18.5	0.17 (4)	10.00			
N-M	0 / 802	-18.5 -18.5	0.26 (4)	10.00			
M-L	0 / 1201	-18.5 -18.5	0.25 (1)	10.00			
L-K	0 / 1201	-18.5 -18.5	0.25 (1)	10.00			
K-J	0 / 815	-18.5 -18.5	0.26 (4)	10.00			
J-I	0 / 0	-18.5 -18.5	0.18 (4)	10.00			
I-H	0 / 0	-18.5 -18.5	0.18 (4)	10.00			

DESIGN CRITERIA			
SPECIFIED LOADS:			
TOP CH.	LL	= 21.0	PSF
	DL	= 6.0	PSF
BOT CH.	LL	= 0.0	PSF
	DL	= 7.4	PSF
TOTAL LOAD = 34.4 PSF			

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE HEEL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (1.00")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI= TC=0.45/1.00 (F-G:1) , BC=0.26/1.00 (J-K:4) , WB=0.35/1.00 (C-M:1) , SSI=0.23/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES					
PLATE	GRIP(DRY)	SHEAR	SECTION		
	(PSI)	(PLI)	(PLI)		
	MAX	MIN	MAX	MIN	MAX
MT20	650	371	1747	788	1987 1873

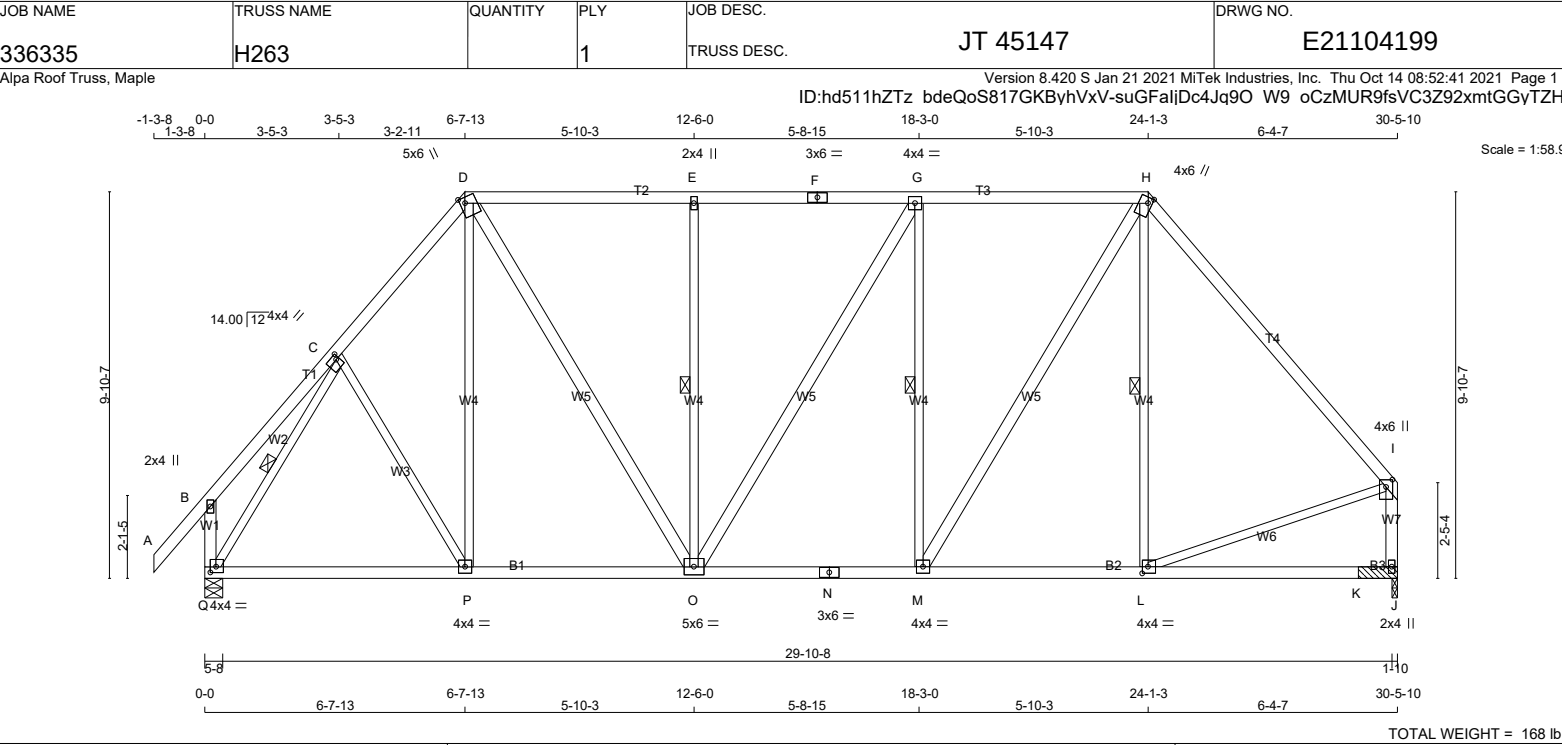
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (F) (INPUT = 0.90)
JSI METAL= 0.48 (G) (INPUT = 1.00)

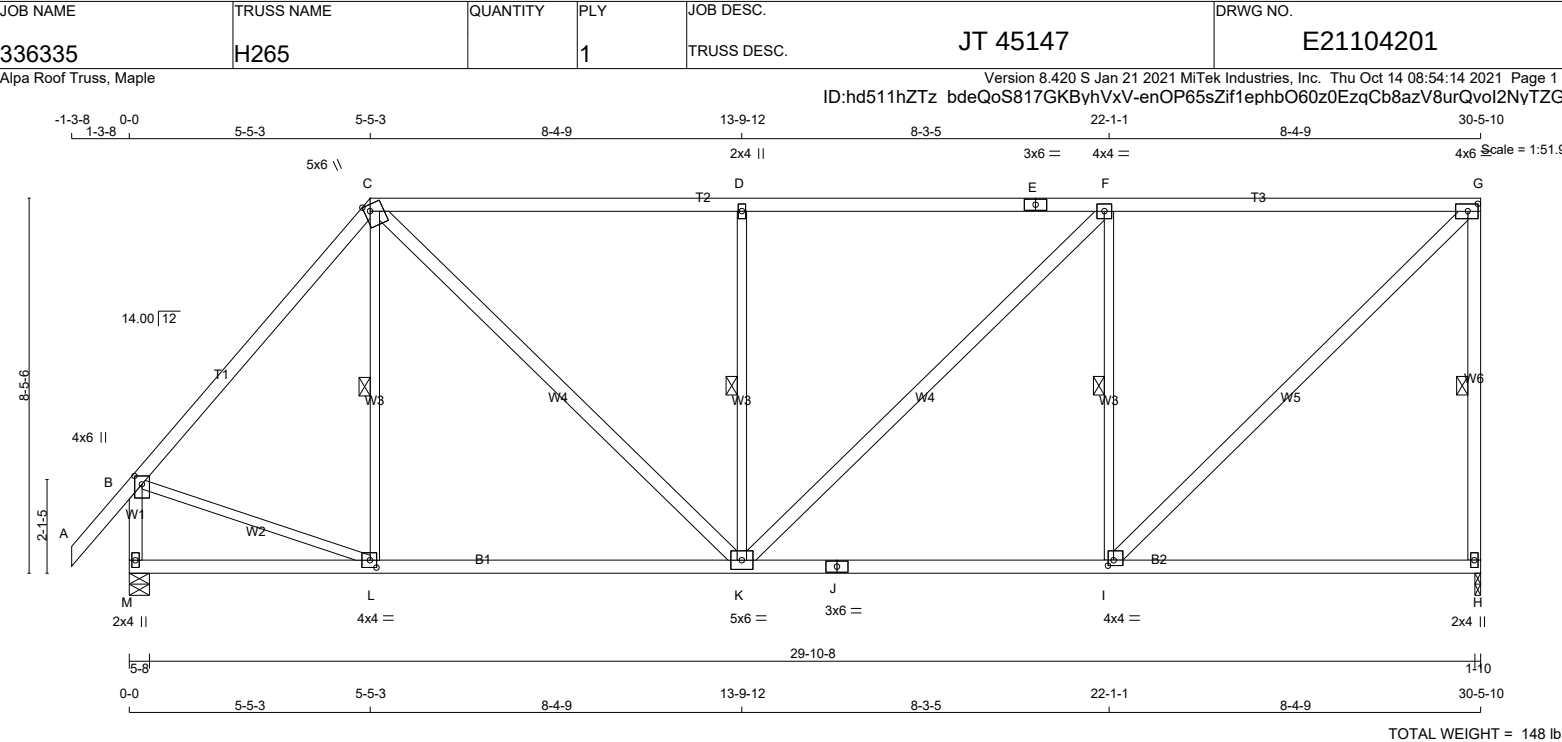
LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2





[illegible]

CONTINUED ON PAGE 2



LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
C - K	2x4	DRY	No.2	SPF
K - F	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
B TMVW+p	MT20	4.0	6.0	2.25 2.00
C TTWW+m	MT20	5.0	6.0	1.75 1.50
D TMW+w	MT20	2.0	4.0	
E TS-t	MT20	3.0	6.0	
F TMWW-t	MT20	4.0	4.0	
G TMVW-t	MT20	4.0	6.0	2.00 2.75
H BMV1+p	MT20	2.0	4.0	
I BMWW-t	MT20	4.0	4.0	1.50 1.50
J BS-t	MT20	3.0	6.0	
K BMWWW-t	MT20	5.0	6.0	
L BMWW-t	MT20	4.0	4.0	2.00 1.75
M BMV1+p	MT20	2.0	4.0	

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQRD					
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX		
H	1470	0	1470	0	0	1-10	1-9		
M	1579	0	1579	0	0	5-8	1-11		

UNFACTORED REACTIONS									
JT	1ST LCASE	MAX./MIN.	COMPONENT	REACTIONS					
H	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
H	1048	640 / 0	0 / 0	0 / 0	0 / 0	408 / 0	0 / 0		
M	1124	698 / 0	0 / 0	0 / 0	0 / 0	425 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.97 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
 ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1 / 2 LENGTH OF G-H, C-L, D-K, F-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING									
TOTAL LOAD CASES: (4)									
C H O R D S									
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM TO				FR-TO			
A- B	0 / 42	-78.0 -78.0	0.11 (1)	10.00	L- C	-114 / 58	0.05 (1)		
B- C	-1268 / 0	-78.0 -78.0	0.53 (1)	5.13	C- K	0 / 859	0.14 (1)		
C- D	-1437 / 0	-78.0 -78.0	0.97 (1)	3.98	K- D	-706 / 0	0.32 (1)		
D- E	-1437 / 0	-78.0 -78.0	0.97 (1)	3.97	K- F	0 / 366	0.06 (1)		
E- F	-1437 / 0	-78.0 -78.0	0.97 (1)	3.97	I- F	-971 / 0	0.44 (1)		
F- G	-1177 / 0	-78.0 -78.0	0.91 (1)	4.35	I- G	0 / 1642	0.26 (1)		
H- G	-1408 / 0	0.0 0.0	0.46 (1)	5.49	B- L	0 / 862	0.19 (1)		
M- B	-1544 / 0	0.0 0.0	0.18 (1)	6.62					
M- L	0 / 0	-18.5 -18.5	0.22 (4)	10.00					
L- K	0 / 821	-18.5 -18.5	0.31 (4)	10.00					
K- J	0 / 1177	-18.5 -18.5	0.43 (4)	10.00					
J- I	0 / 1177	-18.5 -18.5	0.43 (4)	10.00					
I- H	0 / 0	-18.5 -18.5	0.31 (4)	10.00					

DESIGN CRITERIA			
SPECIFIED LOADS:			
TOP CH.	LL = 21.0	PSF	
	DL = 6.0	PSF	
BOT CH.	LL = 0.0	PSF	
	DL = 7.4	PSF	
TOTAL LOAD = 34.4 PSF			

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF BCBC 2018 , ABC 2019
 - PART 9 OF OBC 2012 (2019 AMENDMENT)
 - CSA 086-14
 - TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.02")
 CALCULATED VERT. DEFL.(LL)= L/ 999 (0.06")
 ALLOWABLE DEFL.(TL)= L/360 (1.02")
 CALCULATED VERT. DEFL.(TL)= L/ 999 (0.16")

CSI: TC=0.97/1.00 (C-D:1) , BC=0.43/1.00 (I-K:4) , WB=0.44/1.00 (F-I:1) , SSI=0.30/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

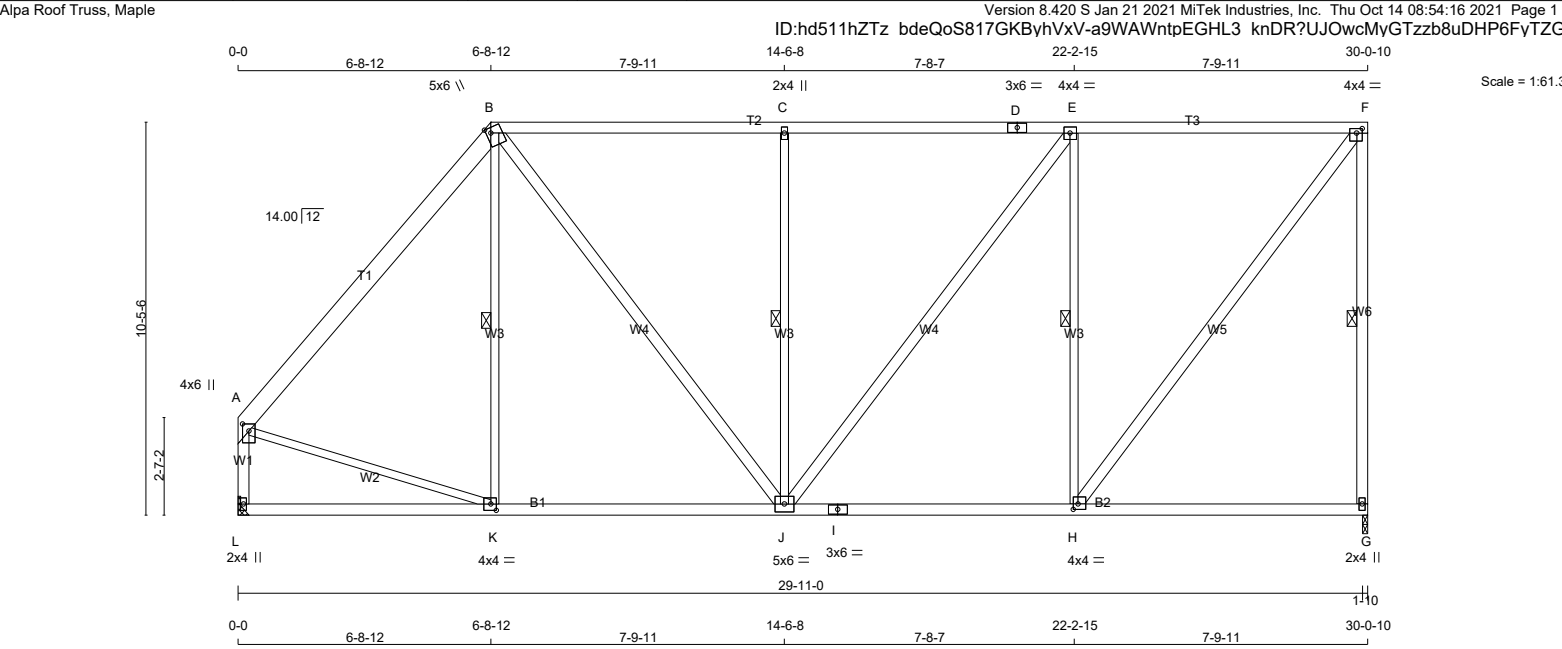
NAIL VALUES					
PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)		
	MAX	MIN	MAX	MIN	MAX
MT20	650	371	1747	788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (B) (INPUT = 0.90)
 JSI METAL= 0.50 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
336335	H266S		1	TRUSS DESC. JT 45147	E21104202



LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - B	2x6	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - A	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
B - J	2x4	DRY	No.2	SPF
J - E	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW+p	MT20	4.0	6.0	2.25 2.00
B	TTWW+m	MT20	5.0	6.0	1.75 1.50
C	TMW+w	MT20	2.0	4.0	
D	TS-t	MT20	3.0	6.0	
E	TMWW-t	MT20	4.0	4.0	
F	TMVW-t	MT20	4.0	4.0	1.50 1.75
G	BMV1+p	MT20	2.0	4.0	
H	BMWW-t	MT20	4.0	4.0	1.75 1.50
I	BS-t	MT20	3.0	6.0	
J	BMWWW-t	MT20	5.0	6.0	
K	BMWW-t	MT20	4.0	4.0	2.00 1.75
L	BMV1+p	MT20	2.0	4.0	

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
	FACTORED		MAXIMUM FACTORED		INPUT		REQRD		
	GROSS REACTION		GROSS REACTION		BRG		BRG		
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX		
G	1450	0	1450	0	0	1-10	1-9		
L	1450	0	1450	0	0	MECHANICAL			
A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT L. MINIMUM BEARING LENGTH AT JOINT L = 1-9.									

UNFACTORED REACTIONS							
	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1033	631 / 0	0 / 0	0 / 0	0 / 0	403 / 0	0 / 0
L	1033	631 / 0	0 / 0	0 / 0	0 / 0	403 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.79 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, B-K, C-J, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING									
TOTAL LOAD CASES: (4)									
CHORDS									
	MAX. FACTORED	FACTORED				WEBS			
	MEMB. FORCE (LBS)	VERT. LOAD	LC1	MAX	MAX.	MEMB. FORCE (LBS)	MAX		
		(PLF)	CSI (LC)	UNBRAC			CSI (LC)		
FR-TO		FROM	TO	LENGTH	FR-TO				
A-B	-1184 / 0	-78.0	-78.0	0.33 (1)	6.25	K-B	-82 / 77	0.06 (1)	
B-C	-1123 / 0	-78.0	-78.0	0.76 (1)	4.80	B-J	0 / 587	0.09 (1)	
C-D	-1124 / 0	-78.0	-78.0	0.76 (1)	4.79	J-C	-649 / 0	0.49 (1)	
D-E	-1124 / 0	-78.0	-78.0	0.76 (1)	4.79	J-E	0 / 400	0.06 (1)	
E-F	-882 / 0	-78.0	-78.0	0.73 (1)	5.29	H-E	-984 / 0	0.75 (1)	
G-F	-1392 / 0	0.0	0.0	0.76 (1)	5.52	H-F	0 / 1447	0.23 (1)	
L-A	-1401 / 0	0.0	0.0	0.18 (1)	6.88	A-K	0 / 800	0.18 (1)	
L-K	0 / 0	-18.5	-18.5	0.24 (4)	10.00				
K-J	0 / 764	-18.5	-18.5	0.31 (4)	10.00				
J-I	0 / 882	-18.5	-18.5	0.36 (4)	10.00				
I-H	0 / 882	-18.5	-18.5	0.36 (4)	10.00				
H-G	0 / 0	-18.5	-18.5	0.27 (4)	10.00				

DESIGN CRITERIA			
SPECIFIED LOADS:			
TOP CH.	LL = 21.0	PSF	
	DL = 6.0	PSF	
BOT CH.	LL = 0.0	PSF	
	DL = 7.4	PSF	
TOTAL LOAD	= 34.4	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")

CALCULATED VERT. DEFL.(LL)= L/ 999 (0.05")

ALLOWABLE DEFL.(TL)= L/360 (1.00")

CALCULATED VERT. DEFL.(TL)= L/ 999 (0.12")

CSI: TC=0.76/1.00 (C-E:1), BC=0.36/1.00 (H-J:4), WB=0.75/1.00 (E-H:1), SSI=0.28/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES					
PLATE	GRIP(DRY)	SHEAR	SECTION		
	(PSI)	(PLI)	(PLI)		
	MAX MIN	MAX MIN	MAX MIN		
MT20	650 371	1747 788	1987 1873		

PLATE PLACEMENT TOL. = 0.250 inches

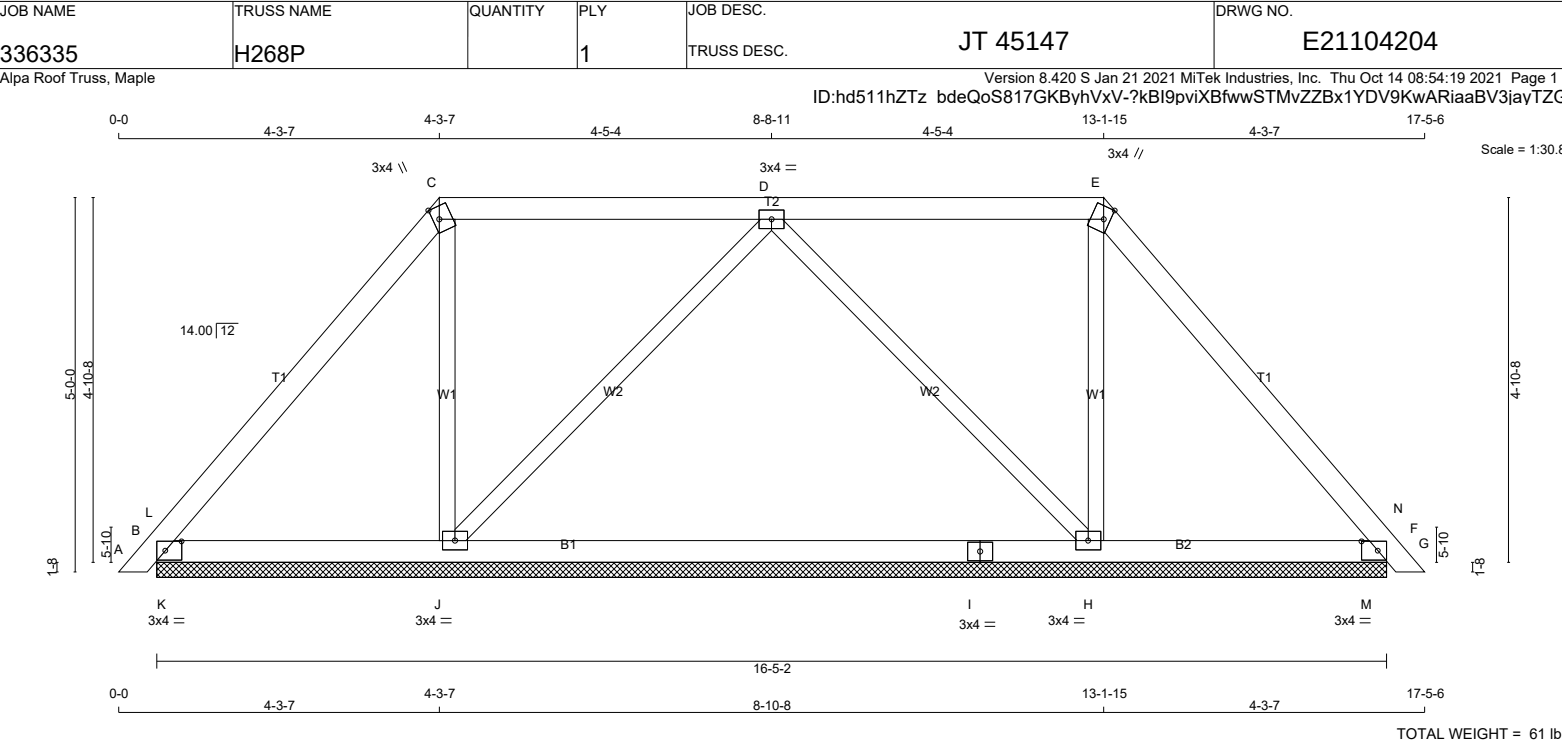
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)

JSI METAL= 0.46 (A) (INPUT = 1.00)



JSI GRIP= 0.74 (J) (INPUT = 0.90)
JSI METAL= 0.12 (D) (INPUT = 1.00)



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
B - I	2x4	DRY	No.2
I - F	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.50
C	TTW+m	MT20	3.0	4.0	2.00	1.00
D	TMWW-t	MT20	3.0	4.0		
E	TTW+m	MT20	3.0	4.0	2.00	1.00
F	TMB1-I	MT20	3.0	4.0	1.50	2.50
H	BMWW1-t	MT20	3.0	4.0		
I	BS-t	MT20	3.0	4.0		
J	BMWW1-t	MT20	3.0	4.0		

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
B	267	0	267	0	16-5-2 (5-5-2)	1-8
J	556	0	556	0	16-5-2 (5-5-2)	1-8
H	556	0	556	0	16-5-2 (5-5-2)	1-8
F	267	0	267	0	16-5-2 (5-5-2)	1-8

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS				
		SNOW	LIVE	PERM.LIVE	WIND	DEAD
B	186	141 / 0	0 / 0	0 / 0	0 / 0	45 / 0
J	401	221 / 0	0 / 0	0 / 0	0 / 0	180 / 0
H	401	221 / 0	0 / 0	0 / 0	0 / 0	180 / 0
F	186	141 / 0	0 / 0	0 / 0	0 / 0	45 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, J, H, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM TO				FR-TO			
A - B	0 / 11	-78.0 -78.0	0.01 (1)	10.00		J - C	-174 / 0	0.06 (1)	
B - L	0 / 215	-78.0 -78.0	0.13 (1)	10.00		J - D	-296 / 0	0.21 (1)	
L - C	-118 / 0	-78.0 -78.0	0.13 (1)	6.25		D - H	-296 / 0	0.21 (1)	
C - D	-62 / 0	-78.0 -78.0	0.26 (1)	6.25		H - E	-174 / 0	0.06 (1)	
D - E	-62 / 0	-78.0 -78.0	0.26 (1)	6.25		K - L	-531 / 0	0.00 (1)	
E - N	-118 / 0	-78.0 -78.0	0.13 (1)	6.25		M - N	-531 / 0	0.00 (1)	
N - F	0 / 215	-78.0 -78.0	0.13 (1)	10.00					
F - G	0 / 11	-78.0 -78.0	0.01 (1)	10.00					
B - K	0 / 66	-18.5 -18.5	0.13 (1)	10.00					
K - J	0 / 66	-18.5 -18.5	0.24 (4)	10.00					
J - I	0 / 266	-18.5 -18.5	0.25 (4)	10.00					
I - H	0 / 266	-18.5 -18.5	0.25 (4)	10.00					
H - M	0 / 66	-18.5 -18.5	0.24 (4)	10.00					
M - F	0 / 66	-18.5 -18.5	0.13 (1)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.26/1.00 (C-D:1) , BC=0.25/1.00 (H-J:4) , WB=0.21/1.00 (D-J:1) , SSI=0.40/1.00 (B-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

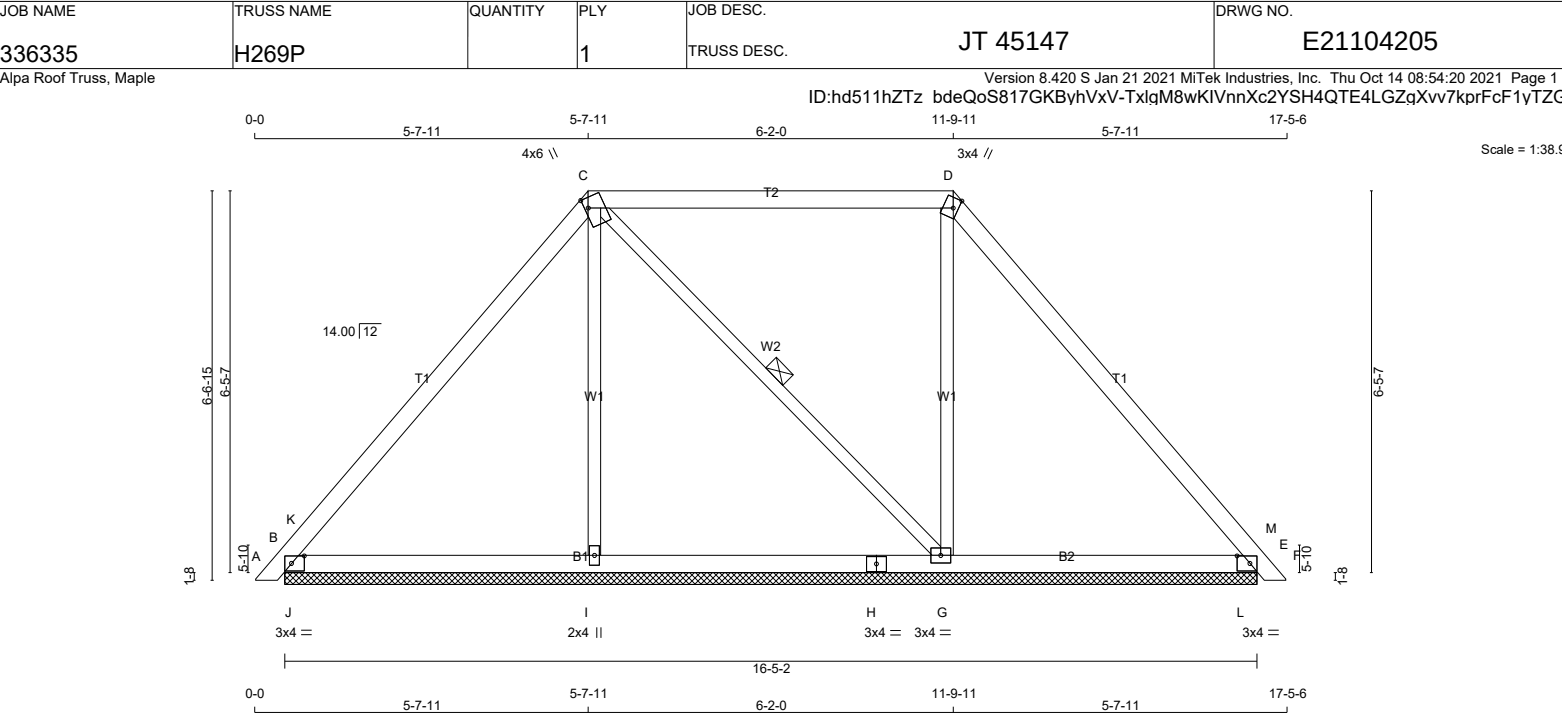
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (J) (INPUT = 0.90)
JSI METAL= 0.09 (I) (INPUT = 1.00)



LUMBER

N. L. G. A. RULES

CHORDS SIZE

A - C2x4 DRY

No.2

SPF

C - D2x4 DRY

No.2

SPF

D - F2x4 DRY

No.2

SPF

B - H2x4 DRY

No.2

SPF

H - E2x4 DRY

No.2

SPF

ALL WEBS2x3 DRY

No.2

SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.50
C	TTWW+m	MT20	4.0	6.0	2.00	0.75
D	TTW+m	MT20	3.0	4.0	2.00	1.00
E	TMB1-I	MT20	3.0	4.0	1.50	2.50
G	BMWV1-t	MT20	3.0	4.0		
H	BS-t	MT20	3.0	4.0		
I	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX
B	502	0	502	0	0	16-5-2 (6-5-2)-8
I	295	0	295	0	0	16-5-2 (6-5-2)-8
G	377	0	377	0	0	16-5-2 (6-5-2)-8
E	472	0	472	0	0	16-5-2 (6-5-2)-8

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS				
	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
B	353	243 / 0	0 / 0	0 / 0	0 / 0	110 / 0	
I	215	104 / 0	0 / 0	0 / 0	0 / 0	111 / 0	
G	271	150 / 0	0 / 0	0 / 0	0 / 0	121 / 0	
E	333	226 / 0	0 / 0	0 / 0	0 / 0	107 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, I, G, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-G.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS					
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM	TO		FR-TO			
A- B	0 / 10	-78.0	-78.0	0.01 (1)	10.00	I- C	-149 / 0	
B- K	-39 / 200	-78.0	-78.0	0.20 (1)	6.25	C- G	-37 / 0	
K- C	-327 / 0	-78.0	-78.0	0.26 (1)	6.25	G- D	-204 / 0	
C- D	-172 / 0	-78.0	-78.0	0.51 (1)	6.25	J- K	-805 / 0	
D- M	-288 / 0	-78.0	-78.0	0.26 (1)	6.25	L- M	-814 / 0	
M- E	-32 / 246	-78.0	-78.0	0.20 (1)	6.25			
E- F	0 / 10	-78.0	-78.0	0.01 (1)	10.00			
B- J	0 / 200	-18.5	-18.5	0.22 (1)	10.00			
J- I	0 / 200	-18.5	-18.5	0.22 (1)	10.00			
I- H	0 / 197	-18.5	-18.5	0.16 (4)	10.00			
H- G	0 / 197	-18.5	-18.5	0.16 (4)	10.00			
G- L	0 / 175	-18.5	-18.5	0.22 (1)	10.00			
L- E	0 / 175	-18.5	-18.5	0.22 (1)	10.00			

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	34.4	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018 , ABC 2019

- PART 9 OF OBC 2012 (2019 AMENDMENT)

- CSA 086-14

- TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.51/1.00 (C-D:1) , BC=0.22/1.00 (I-J:1) , WB=0.14/1.00 (D-G:1) , SSI=0.64/1.00 (E-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

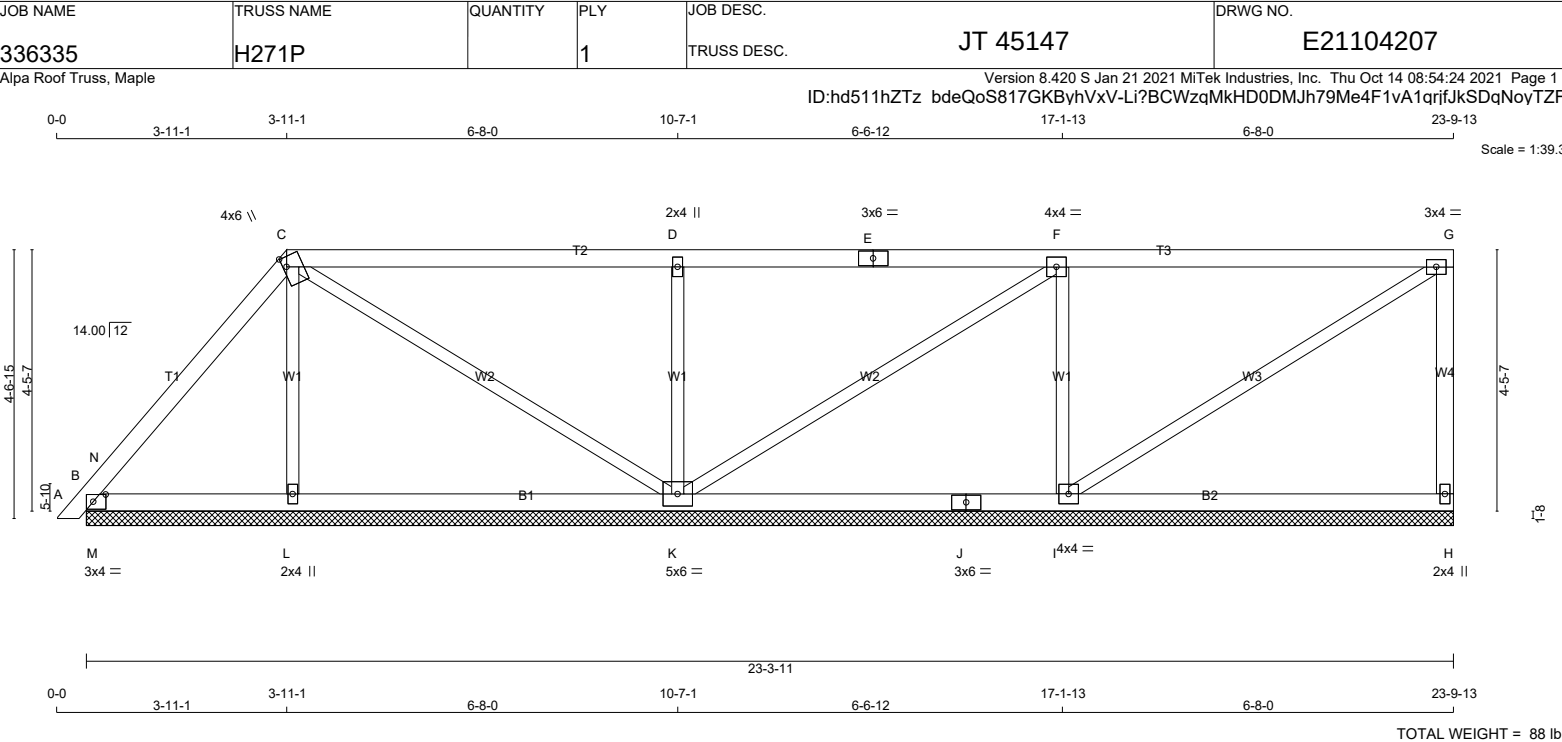
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)

JSI METAL= 0.14 (H) (INPUT = 1.00)



--	--	--



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. A - C 2x4 DRY No.2 SPF C - E 2x4 DRY No.2 SPF E - G 2x4 DRY No.2 SPF H - G 2x4 DRY No.2 SPF B - J 2x4 DRY No.2 SPF J - H 2x4 DRY No.2 SPF ALL WEBS 2x3 DRY No.2 SPF DRY: SEASONED LUMBER.							DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS <table><tr><th rowspan="2">JT</th><th colspan="2">FACTORED GROSS REACTION</th><th colspan="2">MAXIMUM FACTORED GROSS REACTION</th><th rowspan="2">INPUT BRG</th><th colspan="2">REQRD BRG</th></tr><tr><th>VERT</th><th>HORZ</th><th>DOWN</th><th>HORZ</th><th>IN-SX</th><th>IN-SX</th></tr><tr><td>H</td><td>271</td><td>0</td><td>271</td><td>0</td><td>0</td><td>23-3-11 (8-3-11)</td><td></td></tr><tr><td>B</td><td>259</td><td>0</td><td>259</td><td>0</td><td>0</td><td>23-3-11 (8-3-11)</td><td></td></tr><tr><td>L</td><td>316</td><td>0</td><td>316</td><td>0</td><td>0</td><td>23-3-11 (8-3-11)</td><td></td></tr><tr><td>K</td><td>774</td><td>0</td><td>774</td><td>0</td><td>0</td><td>23-3-11 (8-3-11)</td><td></td></tr><tr><td>I</td><td>660</td><td>0</td><td>660</td><td>0</td><td>0</td><td>23-3-11 (8-3-11)</td><td></td></tr></table> UNFACTORED REACTIONS <table><tr><th rowspan="2">JT</th><th colspan="2">1ST LCASE COMBINED</th><th colspan="2">SNOW MAX./MIN. COMPONENT REACTIONS</th><th rowspan="2">WIND</th><th rowspan="2">DEAD</th><th rowspan="2">SOIL</th></tr><tr><th>VERT</th><th>SNOW</th><th>LIVE</th><th>PERM.LIVE</th></tr><tr><td>H</td><td>193</td><td>119 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>73 / 0</td><td>0 / 0</td></tr><tr><td>B</td><td>181</td><td>130 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>51 / 0</td><td>0 / 0</td></tr><tr><td>L</td><td>230</td><td>113 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>117 / 0</td><td>0 / 0</td></tr><tr><td>K</td><td>549</td><td>352 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>197 / 0</td><td>0 / 0</td></tr><tr><td>I</td><td>472</td><td>281 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>191 / 0</td><td>0 / 0</td></tr></table> BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, B, L, K, I BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (4) <table><tr><th colspan="4">CHORDS</th><th colspan="4">WEBS</th></tr><tr><th>MEMB.</th><th>MAX. FACTORED FORCE (LBS)</th><th>FACTORED VERT. LOAD (PLF)</th><th>MAX. LC1 (LC)</th><th>MAX. UNBRAC LENGTH</th><th>MEMB.</th><th>MAX. FACTORED FORCE (LBS)</th><th>MAX. LC1 (LC)</th></tr><tr><td>FR-TO</td><td></td><td>FROM</td><td>TO</td><td></td><td>FR-TO</td><td></td><td></td></tr><tr><td>A- B</td><td>0 / 10</td><td>-78.0</td><td>-78.0</td><td>0.01 (1)</td><td>10.00</td><td>L- C</td><td>-183 / 0</td></tr><tr><td>B- N</td><td>0 / 123</td><td>-78.0</td><td>-78.0</td><td>0.09 (1)</td><td>10.00</td><td>C- K</td><td>-109 / 0</td></tr><tr><td>N- C</td><td>-116 / 0</td><td>-78.0</td><td>-78.0</td><td>0.11 (1)</td><td>6.25</td><td>K- D</td><td>-567 / 0</td></tr><tr><td>C- D</td><td>0 / 31</td><td>-78.0</td><td>-78.0</td><td>0.47 (1)</td><td>10.00</td><td>K- F</td><td>-61 / 0</td></tr><tr><td>D- E</td><td>0 / 30</td><td>-78.0</td><td>-78.0</td><td>0.47 (1)</td><td>10.00</td><td>I- F</td><td>-534 / 0</td></tr><tr><td>E- F</td><td>0 / 30</td><td>-78.0</td><td>-78.0</td><td>0.47 (1)</td><td>10.00</td><td>I- G</td><td>0 / 25</td></tr><tr><td>F- G</td><td>-21 / 0</td><td>-78.0</td><td>-78.0</td><td>0.47 (1)</td><td>6.25</td><td>M- N</td><td>-408 / 0</td></tr><tr><td>H- G</td><td>-222 / 0</td><td>0.0</td><td>0.0</td><td>0.07 (1)</td><td>7.81</td><td></td><td></td></tr><tr><td>B- M</td><td>0 / 67</td><td>-18.5</td><td>-18.5</td><td>0.11 (1)</td><td>10.00</td><td></td><td></td></tr><tr><td>M- L</td><td>0 / 67</td><td>-18.5</td><td>-18.5</td><td>0.14 (4)</td><td>10.00</td><td></td><td></td></tr><tr><td>L- K</td><td>0 / 62</td><td>-18.5</td><td>-18.5</td><td>0.15 (4)</td><td>10.00</td><td></td><td></td></tr><tr><td>K- J</td><td>0 / 21</td><td>-18.5</td><td>-18.5</td><td>0.20 (4)</td><td>10.00</td><td></td><td></td></tr><tr><td>J- I</td><td>0 / 21</td><td>-18.5</td><td>-18.5</td><td>0.20 (4)</td><td>10.00</td><td></td><td></td></tr><tr><td>I- H</td><td>0 / 0</td><td>-18.5</td><td>-18.5</td><td>0.20 (4)</td><td>10.00</td><td></td><td></td></tr></table>							JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG		VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX	H	271	0	271	0	0	23-3-11 (8-3-11)		B	259	0	259	0	0	23-3-11 (8-3-11)		L	316	0	316	0	0	23-3-11 (8-3-11)		K	774	0	774	0	0	23-3-11 (8-3-11)		I	660	0	660	0	0	23-3-11 (8-3-11)		JT	1ST LCASE COMBINED		SNOW MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL	VERT	SNOW	LIVE	PERM.LIVE	H	193	119 / 0	0 / 0	0 / 0	0 / 0	73 / 0	0 / 0	B	181	130 / 0	0 / 0	0 / 0	0 / 0	51 / 0	0 / 0	L	230	113 / 0	0 / 0	0 / 0	0 / 0	117 / 0	0 / 0	K	549	352 / 0	0 / 0	0 / 0	0 / 0	197 / 0	0 / 0	I	472	281 / 0	0 / 0	0 / 0	0 / 0	191 / 0	0 / 0	CHORDS				WEBS				MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)	FR-TO		FROM	TO		FR-TO			A- B	0 / 10	-78.0	-78.0	0.01 (1)	10.00	L- C	-183 / 0	B- N	0 / 123	-78.0	-78.0	0.09 (1)	10.00	C- K	-109 / 0	N- C	-116 / 0	-78.0	-78.0	0.11 (1)	6.25	K- D	-567 / 0	C- D	0 / 31	-78.0	-78.0	0.47 (1)	10.00	K- F	-61 / 0	D- E	0 / 30	-78.0	-78.0	0.47 (1)	10.00	I- F	-534 / 0	E- F	0 / 30	-78.0	-78.0	0.47 (1)	10.00	I- G	0 / 25	F- G	-21 / 0	-78.0	-78.0	0.47 (1)	6.25	M- N	-408 / 0	H- G	-222 / 0	0.0	0.0	0.07 (1)	7.81			B- M	0 / 67	-18.5	-18.5	0.11 (1)	10.00			M- L	0 / 67	-18.5	-18.5	0.14 (4)	10.00			L- K	0 / 62	-18.5	-18.5	0.15 (4)	10.00			K- J	0 / 21	-18.5	-18.5	0.20 (4)	10.00			J- I	0 / 21	-18.5	-18.5	0.20 (4)	10.00			I- H	0 / 0	-18.5	-18.5	0.20 (4)	10.00			DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 21.0 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.4 PSF TOTAL LOAD = 34.4 PSF SPACING = 24.0 IN. C/C LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018 , ABC 2019 - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014 (55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD CSI: TC=0.47/1.00 (D-F:1) , BC=0.20/1.00 (I-K:4) , WB=0.17/1.00 (D-K:1) , SSI=0.31/1.00 (B-M:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10 COMPANION LIVE LOAD FACTOR = 1.00 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT . NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.89 (C) (INPUT = 0.90) JSI METAL= 0.22 (E) (INPUT = 1.00)						
JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG																																																																																																																																																																																																																																																																
	VERT	HORZ	DOWN	HORZ		IN-SX	IN-SX																																																																																																																																																																																																																																																															
H	271	0	271	0	0	23-3-11 (8-3-11)																																																																																																																																																																																																																																																																
B	259	0	259	0	0	23-3-11 (8-3-11)																																																																																																																																																																																																																																																																
L	316	0	316	0	0	23-3-11 (8-3-11)																																																																																																																																																																																																																																																																
K	774	0	774	0	0	23-3-11 (8-3-11)																																																																																																																																																																																																																																																																
I	660	0	660	0	0	23-3-11 (8-3-11)																																																																																																																																																																																																																																																																
JT	1ST LCASE COMBINED		SNOW MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL																																																																																																																																																																																																																																																															
	VERT	SNOW	LIVE	PERM.LIVE																																																																																																																																																																																																																																																																		
H	193	119 / 0	0 / 0	0 / 0	0 / 0	73 / 0	0 / 0																																																																																																																																																																																																																																																															
B	181	130 / 0	0 / 0	0 / 0	0 / 0	51 / 0	0 / 0																																																																																																																																																																																																																																																															
L	230	113 / 0	0 / 0	0 / 0	0 / 0	117 / 0	0 / 0																																																																																																																																																																																																																																																															
K	549	352 / 0	0 / 0	0 / 0	0 / 0	197 / 0	0 / 0																																																																																																																																																																																																																																																															
I	472	281 / 0	0 / 0	0 / 0	0 / 0	191 / 0	0 / 0																																																																																																																																																																																																																																																															
CHORDS				WEBS																																																																																																																																																																																																																																																																		
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)																																																																																																																																																																																																																																																															
FR-TO		FROM	TO		FR-TO																																																																																																																																																																																																																																																																	
A- B	0 / 10	-78.0	-78.0	0.01 (1)	10.00	L- C	-183 / 0																																																																																																																																																																																																																																																															
B- N	0 / 123	-78.0	-78.0	0.09 (1)	10.00	C- K	-109 / 0																																																																																																																																																																																																																																																															
N- C	-116 / 0	-78.0	-78.0	0.11 (1)	6.25	K- D	-567 / 0																																																																																																																																																																																																																																																															
C- D	0 / 31	-78.0	-78.0	0.47 (1)	10.00	K- F	-61 / 0																																																																																																																																																																																																																																																															
D- E	0 / 30	-78.0	-78.0	0.47 (1)	10.00	I- F	-534 / 0																																																																																																																																																																																																																																																															
E- F	0 / 30	-78.0	-78.0	0.47 (1)	10.00	I- G	0 / 25																																																																																																																																																																																																																																																															
F- G	-21 / 0	-78.0	-78.0	0.47 (1)	6.25	M- N	-408 / 0																																																																																																																																																																																																																																																															
H- G	-222 / 0	0.0	0.0	0.07 (1)	7.81																																																																																																																																																																																																																																																																	
B- M	0 / 67	-18.5	-18.5	0.11 (1)	10.00																																																																																																																																																																																																																																																																	
M- L	0 / 67	-18.5	-18.5	0.14 (4)	10.00																																																																																																																																																																																																																																																																	
L- K	0 / 62	-18.5	-18.5	0.15 (4)	10.00																																																																																																																																																																																																																																																																	
K- J	0 / 21	-18.5	-18.5	0.20 (4)	10.00																																																																																																																																																																																																																																																																	
J- I	0 / 21	-18.5	-18.5	0.20 (4)	10.00																																																																																																																																																																																																																																																																	
I- H	0 / 0	-18.5	-18.5	0.20 (4)	10.00																																																																																																																																																																																																																																																																	
PLATES (table is in inches) <table><tr><th>JT</th><th>TYPE</th><th>PLATES</th><th>W</th><th>LEN</th><th>Y</th><th>X</th></tr><tr><td>B</td><td>TMB1-I</td><td>MT20</td><td>3.0</td><td>4.0</td><td>1.50</td><td>2.50</td></tr><tr><td>C</td><td>TTWW+m</td><td>MT20</td><td>4.0</td><td>6.0</td><td>2.00</td><td>0.75</td></tr><tr><td>D</td><td>TMW+w</td><td>MT20</td><td>2.0</td><td>4.0</td><td></td><td></td></tr><tr><td>E</td><td>TS-t</td><td>MT20</td><td>3.0</td><td>6.0</td><td></td><td></td></tr><tr><td>F</td><td>TMWW-t</td><td>MT20</td><td>4.0</td><td>4.0</td><td></td><td></td></tr><tr><td>G</td><td>TMVW-t</td><td>MT20</td><td>3.0</td><td>4.0</td><td></td><td></td></tr><tr><td>H</td><td>BMV1+p</td><td>MT20</td><td>2.0</td><td>4.0</td><td></td><td></td></tr><tr><td>I</td><td>BMWW1-t</td><td>MT20</td><td>4.0</td><td>4.0</td><td></td><td></td></tr><tr><td>J</td><td>BS-t</td><td>MT20</td><td>3.0</td><td>6.0</td><td></td><td></td></tr><tr><td>K</td><td>BMWWW1-t</td><td>MT20</td><td>5.0</td><td>6.0</td><td></td><td></td></tr><tr><td>L</td><td>BMW1+w</td><td>MT20</td><td>2.0</td><td>4.0</td><td></td><td></td></tr></table>							JT	TYPE	PLATES	W	LEN	Y	X	B	TMB1-I	MT20	3.0	4.0	1.50	2.50	C	TTWW+m	MT20	4.0	6.0	2.00	0.75	D	TMW+w	MT20	2.0	4.0			E	TS-t	MT20	3.0	6.0			F	TMWW-t	MT20	4.0	4.0			G	TMVW-t	MT20	3.0	4.0			H	BMV1+p	MT20	2.0	4.0			I	BMWW1-t	MT20	4.0	4.0			J	BS-t	MT20	3.0	6.0			K	BMWWW1-t	MT20	5.0	6.0			L	BMW1+w	MT20	2.0	4.0			LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2																																																																																																																																																																											
JT	TYPE	PLATES	W	LEN	Y	X																																																																																																																																																																																																																																																																
B	TMB1-I	MT20	3.0	4.0	1.50	2.50																																																																																																																																																																																																																																																																
C	TTWW+m	MT20	4.0	6.0	2.00	0.75																																																																																																																																																																																																																																																																
D	TMW+w	MT20	2.0	4.0																																																																																																																																																																																																																																																																		
E	TS-t	MT20	3.0	6.0																																																																																																																																																																																																																																																																		
F	TMWW-t	MT20	4.0	4.0																																																																																																																																																																																																																																																																		
G	TMVW-t	MT20	3.0	4.0																																																																																																																																																																																																																																																																		
H	BMV1+p	MT20	2.0	4.0																																																																																																																																																																																																																																																																		
I	BMWW1-t	MT20	4.0	4.0																																																																																																																																																																																																																																																																		
J	BS-t	MT20	3.0	6.0																																																																																																																																																																																																																																																																		
K	BMWWW1-t	MT20	5.0	6.0																																																																																																																																																																																																																																																																		
L	BMW1+w	MT20	2.0	4.0																																																																																																																																																																																																																																																																		
																																																																																																																																																																																																																																																																						

Scale = 1:40.9

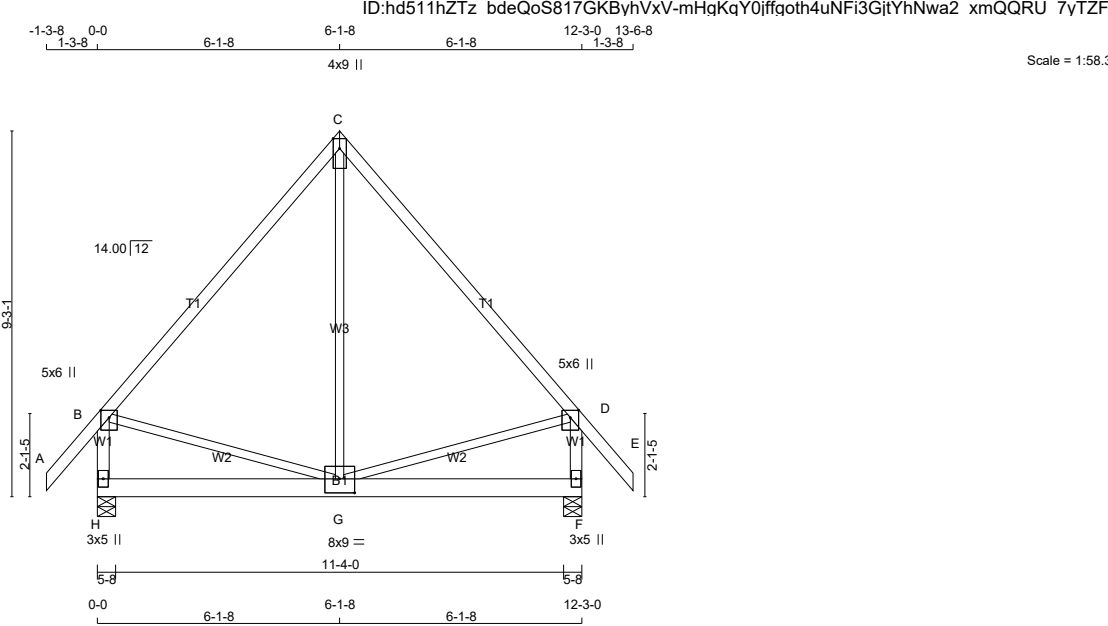


JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.18 (E) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
336335	H273		2	TRUSS DESC. JT 45147	E21104209

Alpa Roof Truss, Maple

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Oct 14 08:54:27 2021 Page 1



TOTAL WEIGHT = 2 X 69 = 137 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - C	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x6	DRY	1650F 1.5E	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF **2** TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	1	12
C - E	1	12
H - B	1	12
F - D	1	12
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
H - F	2	8
WEBS : (0.122"x3") SPIRAL NAILS		SIDE(338.3)
2x3	1	6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.25	2.50
C	TTW+p	MT20	4.0	9.0	3.00	Edge
D	TMVW+p	MT20	5.0	6.0	2.25	2.50
F	BMV1+p	MT20	3.0	5.0		
G	BMWWW-t	MT20	8.0	9.0	4.25	4.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX		REQRD BRG IN-SX	
JT	VERT	HORZ		DOWN	HORZ	UPLIFT			
H	4844	0		4844	0	0	5-8	4-6	
F	4844	0		4844	0	0	5-8	4-6	

UNFACTORED REACTIONS

		1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
H	3451	2119 / 0	0 / 0	0 / 0	0 / 0	1332 / 0	0 / 0		
F	3451	2119 / 0	0 / 0	0 / 0	0 / 0	1332 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.91 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.		MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH	MEMB.		MAX. FACTORED FORCE (LBS)
FR-TO			FROM TO		FR-TO		
A-B		0 / 42	-78.0 -78.0	0.06 (1)	G-C		0 / 4173
B-C		-3062 / 0	-78.0 -78.0	0.50 (1)	B-G		0 / 2065
C-D		-3062 / 0	-78.0 -78.0	0.50 (1)	G-D		0 / 2065
D-E		0 / 42	-78.0 -78.0	0.06 (1)			
H-B		-3212 / 0	0.0 0.0	0.20 (1)			
F-D		-3212 / 0	0.0 0.0	0.20 (1)			
H-G		0 / 0	-695.0 -695.0	0.77 (1)			
G-F		0 / 0	-695.0 -695.0	0.77 (1)			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	34.4	PSF	

SPACING = 24.0 IN./C

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 30-0-10
END DISTANCE = 12-3-0
END SPAN CARRIED = 30-0-10
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDTL LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 882 (0.17")

CSI: TC=0.50/1.00 (B-C:1) , BC=0.77/1.00 (G-H:1) , WB=0.52/1.00 (C-G:1) , SSI=0.87/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

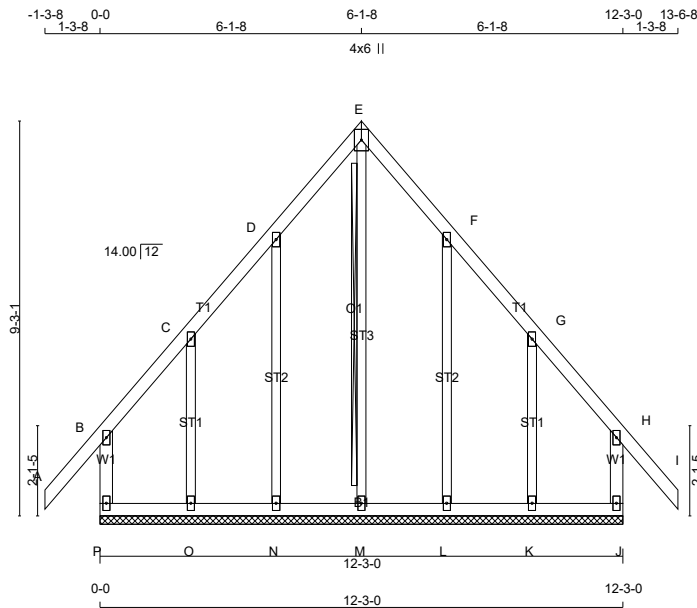
JSI GRIP= 0.81 (B) (INPUT = 0.90)
JSI METAL= 0.44 (D) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



Alpa Roof Truss, Maple

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Oct 14 08:54:29 2021 Page 1
ID:hd511hZTz bdeQoS817GKByhVxV-ifo4FD1zAGwV6 EHUgkXL8y FBnrWzP2tkwb3?yTZFu



Scale = 1:54.0

TOTAL WEIGHT = 68 lb

N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
P - B	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

GABLE STUDS SPACED AT 2'-0" OC.

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C, D, F, G						
C	TMW+w	MT20	2.0	4.0		
E	TTW+p	MT20	4.0	6.0		
H	TMV+p	MT20	2.0	4.0		
J	BMV1+p	MT20	2.0	4.0		
K, L, M, N, O						
K	BMV1+w	MT20	2.0	4.0		
P	BMV1+p	MT20	2.0	4.0		

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED

2x3 DRY SPF No.2 T-BRACE AT E-M

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM TO		LENGTH	FR-TO			
P-B	-208 / 0	0.0	0.0	0.02 (1)	7.81	M-E	-145 / 0	0.20 (1)
A-B	0 / 42	-78.0	-78.0	0.11 (1)	10.00	N-D	-158 / 0	0.13 (1)
B-C	-25 / 0	-78.0	-78.0	0.04 (1)	6.25	O-C	-149 / 0	0.05 (1)
C-D	-17 / 0	-78.0	-78.0	0.04 (1)	6.25	L-F	-158 / 0	0.13 (1)
D-E	-13 / 0	-78.0	-78.0	0.04 (1)	6.25	K-G	-149 / 0	0.05 (1)
E-F	-13 / 0	-78.0	-78.0	0.04 (1)	6.25			
F-G	-17 / 0	-78.0	-78.0	0.04 (1)	6.25			
G-H	-25 / 0	-78.0	-78.0	0.04 (1)	6.25			
H-I	0 / 42	-78.0	-78.0	0.11 (1)	10.00			
J-H	-208 / 0	0.0	0.0	0.02 (1)	7.81			
P-O	0 / 16	-18.5	-18.5	0.02 (1)	10.00			
O-N	0 / 12	-18.5	-18.5	0.02 (4)	10.00			
N-M	0 / 9	-18.5	-18.5	0.02 (4)	10.00			
M-L	0 / 9	-18.5	-18.5	0.02 (4)	10.00			
L-K	0 / 12	-18.5	-18.5	0.02 (4)	10.00			
K-J	0 / 16	-18.5	-18.5	0.02 (1)	10.00			

SPECIFIED LOADS:				
TOP	CH.	LL =	21.0	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL LOAD		=	34.4	PSF

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11/1.00 (H-I:1) , BC=0.02/1.00 (J-K:1) ,
WB=0.20/1.00 (E-M:1) , SSI=0.06/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

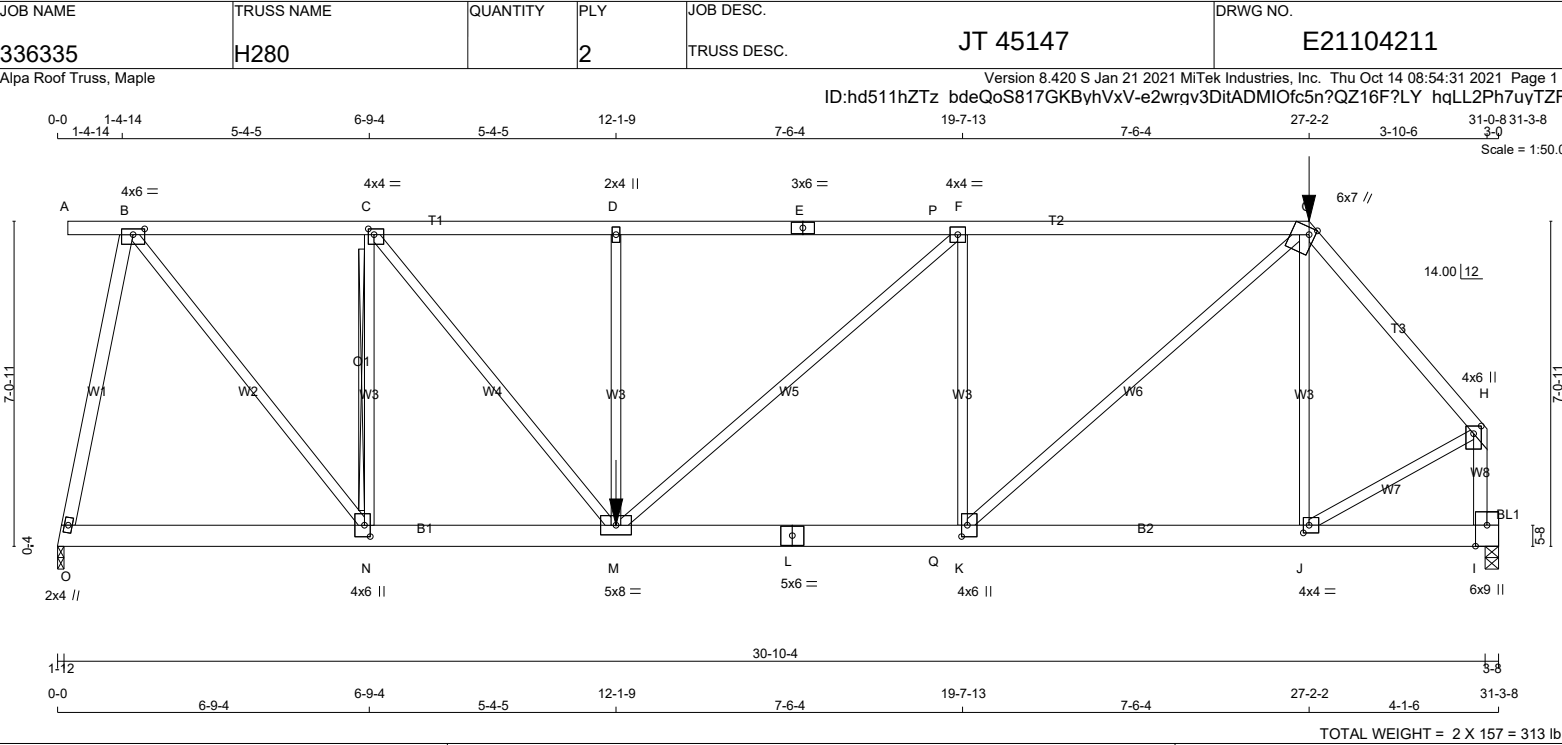
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (B) (INPUT = 0.90)
JSI METAL= 0.11 (B) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE
2X4 SPF#2





LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - E	2x4	DRY No.2	SPF
E - G	2x4	DRY No.2	SPF
G - H	2x4	DRY No.2	SPF
I - H	2x4	DRY No.2	SPF
O - L	2x6	DRY No.2	SPF
L - I	2x6	DRY No.2	SPF

BEARING BLOCKS			
BL1	2x4	DRY No.2	SPF
ALL WEBS EXCEPT			
O - B	2x4	DRY No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - E	12	TOP
E - G	12	SIDE(43.9)
G - H	12	SIDE(61.0)
I - H	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
O - L	12	SIDE(131.2)
L - I	12	SIDE(66.3)
WEBS : (0.122"x3") SPIRAL NAILS		
J - G	6	SIDE(25.9)
D - M	6	SIDE(269.2)
2x3	6	
2x4	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
O	3102	0	3102	0	1-12
I	3492	0	3492	0	3-8

UNFACTORED REACTIONS		1ST LCASE	MAX./MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
O	2212	1351 / 0	0 / 0
I	2491	1514 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.42 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT C-N

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING		TOTAL LOAD CASES: (4)			
CHORDS		MAX. FACTORED		WEBS	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)
FR-TO		FROM	TO	FR-TO	
A - B	0 / 0	-78.0	-78.0 0.05 (1)	O - B	-3097 / 0
B - C	-2766 / 0	-78.0	-78.0 0.22 (1)	B - N	0 / 3530
C - D	-4581 / 0	-78.0	-78.0 0.30 (1)	N - C	-2736 / 0
D - E	-4581 / 0	-78.0	-78.0 0.97 (1)	C - M	0 / 2900
E - P	-4581 / 0	-78.0	-78.0 0.97 (1)	M - D	-395 / 0
P - F	-4581 / 0	-165.7	-165.7 0.97 (1)	D - F	0 / 419
F - G	-4267 / 0	-165.7	-165.7 0.93 (1)	K - F	-1420 / 0
G - H	-2860 / 0	-78.0	-78.0 0.16 (1)	K - G	0 / 3242
H - I	-3441 / 0	0.0	0.0 0.24 (1)	J - G	-922 / 0
				J - H	0 / 2364
O - N	0 / 587	-18.5	-18.5 0.08 (1)		
N - M	0 / 2766	-18.5	-18.5 0.31 (1)		
M - L	0 / 4268	-151.1	-151.1 0.52 (1)		
L - Q	0 / 4268	-151.1	-151.1 0.52 (1)		
Q - K	0 / 4268	-39.3	-39.3 0.52 (1)		
K - J	0 / 1838	-39.3	-39.3 0.29 (1)		
J - I	-239 / 0	-39.3	-39.3 0.07 (4)		

FACTORED CONCENTRATED LOADS (LBS)		LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	27-2-2	-267	-267	---	---	FRONT	VERT	TOTAL	---	C1
M	12-1-9	-1480	-1480	---	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:		TOP CH.	LL	PSF
		DL	6.0	PSF
BOT CH.		LL	0.0	PSF
		DL	7.4	PSF
TOTAL LOAD		= 34.4 PSF		

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CStdGirder
START DISTANCE = 12-1-9
START SPAN CARRIED = 7-6-0
END DISTANCE = 19-1-0
END SPAN CARRIED = 7-6-0
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 1-4-14
RIGHT SETBACK = 4-1-6
END SETBACK = 6-6-0
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 12-2-8 OF SPAN MEASURED FROM THE RIGHT.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.02")
CALCULATED VERT. DEFL.(LL) = L/999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (1.02")
CALCULATED VERT. DEFL.(TL) = L/999 (0.25")

CSI: TC=0.97/1.00 (D-F:1), BC=0.52/1.00 (K-M:1), WB=0.97/1.00 (B-O:1), SSI=0.31/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
336335	H280		2	JT 45147	E21104211(2)

Alpa Roof Truss, Maple

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Oct 14 08:54:31 2021 Page 2

ID:hd511hZTz_bdeQoS817GKByhVxV-e2wrgv3DiADMIOfc5n?QZ16F?LY_hqLL2Ph7uyTZFs

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	4.0	6.0	1.50	3.00
C	TMWW-t	MT20	4.0	4.0	1.50	1.50
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTWW+m	MT20	6.0	7.0	Edge	1.50
H	TMVW+p	MT20	4.0	6.0	2.00	2.00
I	BMVK1+p	MT20	6.0	9.0	Edge	3.00
J	BMWW-t	MT20	4.0	4.0	2.00	1.50
K	BMWW+t	MT20	4.0	6.0	3.00	1.50
L	BS-t	MT20	5.0	6.0		
M	BMWWW-t	MT20	5.0	8.0		
N	BMWW+t	MT20	4.0	6.0	3.00	1.50
O	BMW1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.67 (L) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE
2X4 SPF#2



The diagram illustrates a roof truss system with the following dimensions and member labels:

- Horizontal Dimensions (Top):** 1-3-8, 4-2-14, 6-1-1, 10-3-15, 16-5-0, 18-5-0, 23-1-0, 27-9-0, 29-9-0, 30-0-2, 31-5-0. Total length: 152'-0".
- Horizontal Dimensions (Bottom):** 5-8, 4-2-14, 4-2-14, 6-1-1, 10-3-15, 6-1-1, 16-5-0, 2-0-0, 18-5-0, 4-8-0, 23-1-0, 4-8-0, 27-9-0, 2-0-0, 29-9-0, 1-8-0, 31-5-0. Total length: 152'-0".
- Vertical Dimensions (Right):** 7'-0-11, 1'-4-0, 0'-4".
- Truss Members:**
 - Top Chords:** T1, T2, T3
 - Bottom Chords:** B1, B2, B3, B5, B6
 - Vertical Members:** W1, W2, W3, W4, W5, W6, W7, W8, W9, W10
 - Diagonal Members:** D1, D2
 - Other Members:** U, V, X, Y, Z
- Supports:**
 - Left Support:** Pin support at node U, roller support at node B.
 - Right Support:** Roller support at node L.
- Scale:** 1:52.7

LUMBER				
N. L. G. A. RULES				
CHORDS		SIZE	LUMBER	DESCR
A - C	2x4	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - K	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
U - S	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - P	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
C- G	1	12	SIDE(43.9)
G- K	1	12	SIDE(44.7)
U- B	2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS			
U- S	1	12	SIDE(11.7)
S- Q	1	12	SIDE(11.7)
Q- P	1	12	SIDE(45.2)
P- N	1	12	SIDE(54.3)
N- M	1	12	SIDE(45.3)
M- L	1	12	SIDE(11.7)
WEBS : (0.122"x3") SPIRAL NAILS			
T- C	1	6	SIDE(28.1)
2x3	1	6	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

LATERAL BRACE(S) SHOWN SHALL BE
2X4 SPF#2



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
L	3216	0	3216	0	0	2-0	1-12
U	3196	0	3196	0	0	5-8	1-12

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	2293	1401 / 0	0 / 0	0 / 0	0 / 0	892 / 0	0 / 0
U	2279	1391 / 0	0 / 0	0 / 0	0 / 0	888 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, U

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.08 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT J-L, E-Q

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MAX. FACTORED		FACTORED			MAX. FACTORED				
MEMB.	FORCE	VERT.	LOAD	LC1	MAX	MAX.	MEMB.	MAX.	FORCE
	(LBS)	(PLF)			CSI (LC)	UNBRAC			MAX
FR-TO		FROM	TO			LENGTH	FR-TO		CSI (LC)
A-B	0 / 42	-78.0	-78.0	0.06	(1)	10.00	J-L	-3271 / 0	0.64 (1)
B-C	-2800 / 0	-78.0	-78.0	0.19	(1)	5.30	T-C	-506 / 0	0.22 (1)
C-D	-3418 / 0	-165.7	-165.7	0.63	(1)	4.27	C-R	0 / 2402	0.30 (1)
D-E	-3757 / 0	-165.7	-165.7	0.65	(1)	4.08	R-D	-1524 / 0	0.66 (1)
E-F	-4478 / 0	-78.0	-78.0	0.30	(1)	4.22	Q-Q	0 / 505	0.06 (1)
F-G	-4479 / 0	-78.0	-78.0	0.23	(1)	4.35	Q-E	-2620 / 0	0.59 (1)
G-H	-4479 / 0	-78.0	-78.0	0.23	(1)	4.35	E-P	0 / 2103	0.26 (1)
H-I	-3645 / 0	-78.0	-78.0	0.20	(1)	4.76	P-F	-85 / 0	0.02 (1)
I-J	-1830 / 0	-78.0	-78.0	0.16	(1)	6.23	P-H	0 / 1278	0.16 (1)
J-K	0 / 0	-167.5	-167.5	0.15	(1)	10.00	O-H	-1398 / 0	0.36 (1)
U-B	-3134 / 0	0.0	0.0	0.13	(1)	7.80	O-I	0 / 2756	0.34 (1)
							N-I	-2245 / 0	0.58 (1)
U-T	0 / 0	-39.3	-39.3	0.12	(4)	10.00	B-T	0 / 1954	0.24 (1)
T-S	0 / 1806	-39.3	-39.3	0.29	(4)	10.00	J-M	-329 / 0	0.14 (1)
S-R	0 / 1806	-39.3	-39.3	0.29	(4)	10.00	N-J	0 / 3282	0.41 (1)
R-Q	0 / 3418	-39.3	-39.3	0.40	(1)	10.00			
Q-P	0 / 4466	-127.0	-127.0	0.44	(1)	10.00			
P-O	0 / 3645	-127.0	-127.0	0.56	(1)	10.00			
O-N	0 / 1873	-127.0	-127.0	0.40	(1)	10.00			
N-M	0 / 883	-127.0	-127.0	0.13	(1)	10.00			
M-L	0 / 731	-39.3	-39.3	0.07	(1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	4-2-14	-275	-275	---	FRONT	VERT	TOTAL	---	C1

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP	CH.	LL =	21.0	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 4-2-14
RIGHT SETBACK = 0-0
END SETBACK = 6-6-0
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADD'T'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 16-5-0 OF SPAN
MEASURED FROM THE LEFT.

GIRDER TYPE: CStdGirder
START DISTANCE = 16-5-0
START SPAN CARRIED = 6-6-0
END DISTANCE = 29-9-0
END SPAN CARRIED = 6-6-0
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADD'L LOADS BASED ON 55 % OF GSL.

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 4-2-14
RIGHT SETBACK = 0-0
END SETBACK = 6-6-0
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADD'TL LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 1-8-0 OF SPAN
MEASURED FROM THE RIGHT.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL
LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
336335	H280T		2	JT 45147	E21104212(2)

Alpa Roof Truss, Maple

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Oct 14 08:54:33 2021 Page 2

ID:hd511hZTz_bdeQoS817GKByhVxV-aR2b5b4UEVQxbcY2jWpTV_7Wso1ISg6eoMuoCmyTZFq

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	9.0	2.00	3.50
C	TTWW+m	MT20	5.0	6.0	1.75	1.50
D	TMWW-t	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0	1.50	2.00
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0		
I	TMWW-t	MT20	4.0	4.0	1.50	1.75
J	TMWWW-t	MT20	5.0	8.0	1.75	4.00
L	BMW1+w	MT20	2.0	4.0		
M	BBW-l	MT20	5.0	6.0	3.00	4.25
N	BBWW-l	MT20	7.0	8.0	3.00	4.00
O	BMWW-t	MT20	4.0	4.0	1.50	1.75
P	BBWWW-p	MT20	6.0	7.0	3.00	4.00
Q	BBWW-h	MT20	6.0	7.0	2.25	4.25
R	BMWW-t	MT20	4.0	4.0	1.75	1.75
S	BS-t	MT20	3.0	6.0		
T	BMWW-t	MT20	4.0	4.0	2.00	1.75
U	BMV1+p	MT20	2.0	4.0		

ALLOWABLE DEFL.(LL)= L/360 (1.04")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (1.04")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.65/1.00 (D-E:1) , BC=0.56/1.00 (O-P:1) ,
WB=0.66/1.00 (D-R:1) , SSI=0.26/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN THE
TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.42 (Q) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE
2X4 SPF#2



The diagram illustrates a roof truss system with the following details:

- Members:**
 - Top Chords: T1, T2, T3
 - Bottom Chords: B1, B2
 - Verticals: V1, V2, V3, V4
 - Diagonals: W1, W2, W3, W4, W5, W6
- Joints:** A, B, C, D, E, F, G, H, I, J, K, L, M
- Dimensions:**
 - Horizontal: 1-7-10, 8-2-10, 9-10-5, 8-1-6, 17-11-11, 8-2-10, 26-2-5, 4-11-5, 31-1-10
 - Vertical: 14.00, 11.12, 2.5-4, 8-2-7
- Scale:** 1:50

JSI GRIP= 0.85 (J) (INPUT = 0.90)
JSI METAL= 0.46 (G) (INPUT = 1.00)

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Oct 14 08:54:37 2021 Page 1



TOTAL WEIGHT = 169 lb

DRY: SEASONED LUMBER.

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWWw-t	MT20	5.0	8.0	1.50	4.00
C	TMWW-t	MT20	4.0	4.0	1.75	1.75
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0	1.50	2.00
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW+m	MT20	5.0	6.0	1.75	1.50
J	TMWv+p	MT20	4.0	6.0	2.25	2.00
K	BMV1+p	MT20	2.0	4.0		
L	BMWW-t	MT20	4.0	4.0	2.00	1.75
M	BS-t	MT20	3.0	6.0		
N	BMWW-t	MT20	4.0	4.0	2.00	1.75
O	BBWW-h	MT20	6.0	7.0	2.25	4.25
P	BBWWw-m	MT20	6.0	7.0	3.00	3.00
Q	BMWW-t	MT20	4.0	4.0	1.75	1.75
R	BBWW-l	MT20	7.0	8.0	3.00	4.00
S	BBW-l	MT20	5.0	6.0	3.00	4.25
T	BMW1+w	MT20	2.0	4.0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, K

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.79 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

TOTAL LOAD CASES: (4)

CHORDS					WEB S				
MAX. FACTORED		FACTORED		MAX. UNBRAC	MAX. FACTORED		MAX. CSI (LC)		
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)		MEMB.	FORCE (LBS)			
FR-TO		FROM	TO	LENGTH	FR-TO				
A-B	0 / 0	-78.0	-78.0	0.16 (1)	10.00	T-B	-1514 / 0	0.49 (1)	
B-C	-706 / 0	-78.0	-78.0	0.17 (1)	6.25	R-C	-1141 / 0	0.91 (1)	
C-D	-1391 / 0	-78.0	-78.0	0.27 (1)	5.21	C-Q	0 / 1170	0.26 (1)	
D-E	-1716 / 0	-78.0	-78.0	0.29 (1)	4.79	Q-D	-865 / 0	0.69 (1)	
E-F	-1712 / 0	-78.0	-78.0	0.20 (1)	4.88	D-P	0 / 560	0.13 (1)	
F-G	-1476 / 0	-78.0	-78.0	0.43 (1)	4.88	P-E	-208 / 0	0.17 (1)	
G-H	-1476 / 0	-78.0	-78.0	0.43 (1)	4.88	P-F	0 / 822	0.18 (1)	
H-I	-1350 / 0	-78.0	-78.0	0.42 (1)	5.05	O-F	-1087 / 0	0.46 (1)	
I-J	-1229 / 0	-78.0	-78.0	0.42 (1)	5.32	O-H	0 / 217	0.05 (1)	
K-J	-1455 / 0	0.0	0.0	0.19 (1)	6.77	N-H	-674 / 0	0.88 (1)	
						N-I	0 / 960	0.22 (1)	
T-S	0 / 333	-18.5	-18.5	0.06 (1)	10.00	L-I	-219 / 0	0.29 (1)	
S-R	0 / 410	-18.5	-18.5	0.08 (1)	10.00	L-J	0 / 861	0.19 (1)	
R-Q	0 / 724	-18.5	-18.5	0.18 (4)	10.00	S-B	-194 / 0	0.26 (1)	
Q-P	0 / 1391	-18.5	-18.5	0.28 (1)	10.00	B-R	0 / 1468	0.33 (1)	
P-O	0 / 1756	-18.5	-18.5	0.29 (1)	10.00				
O-N	0 / 1350	-18.5	-18.5	0.29 (1)	10.00				
N-M	0 / 794	-18.5	-18.5	0.21 (4)	10.00				
M-L	0 / 794	-18.5	-18.5	0.21 (4)	10.00				
L-K	0 / 0	-18.5	-18.5	0.12 (4)	10.00				

SPECIFIED LOADS:

TOP	CH.	LL	=	21.0	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.4	PSF
TOTAL LOAD				=	34.4 PSF

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= $L/360$ (1.03")
 CALCULATED VERT. DEFL.(LL) = $L/999$ (0.09")
 ALLOWABLE DEFL.(TL)= $L/360$ (1.03")
 CALCULATED VERT. DEFL.(TL) = $L/999$ (0.18")

CSI: TC=0.43/1.00 (F-H:1) , BC=0.29/1.00 (O-P:1) ,
WB=0.91/1.00 (C-R:1) , SSI=0.21/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

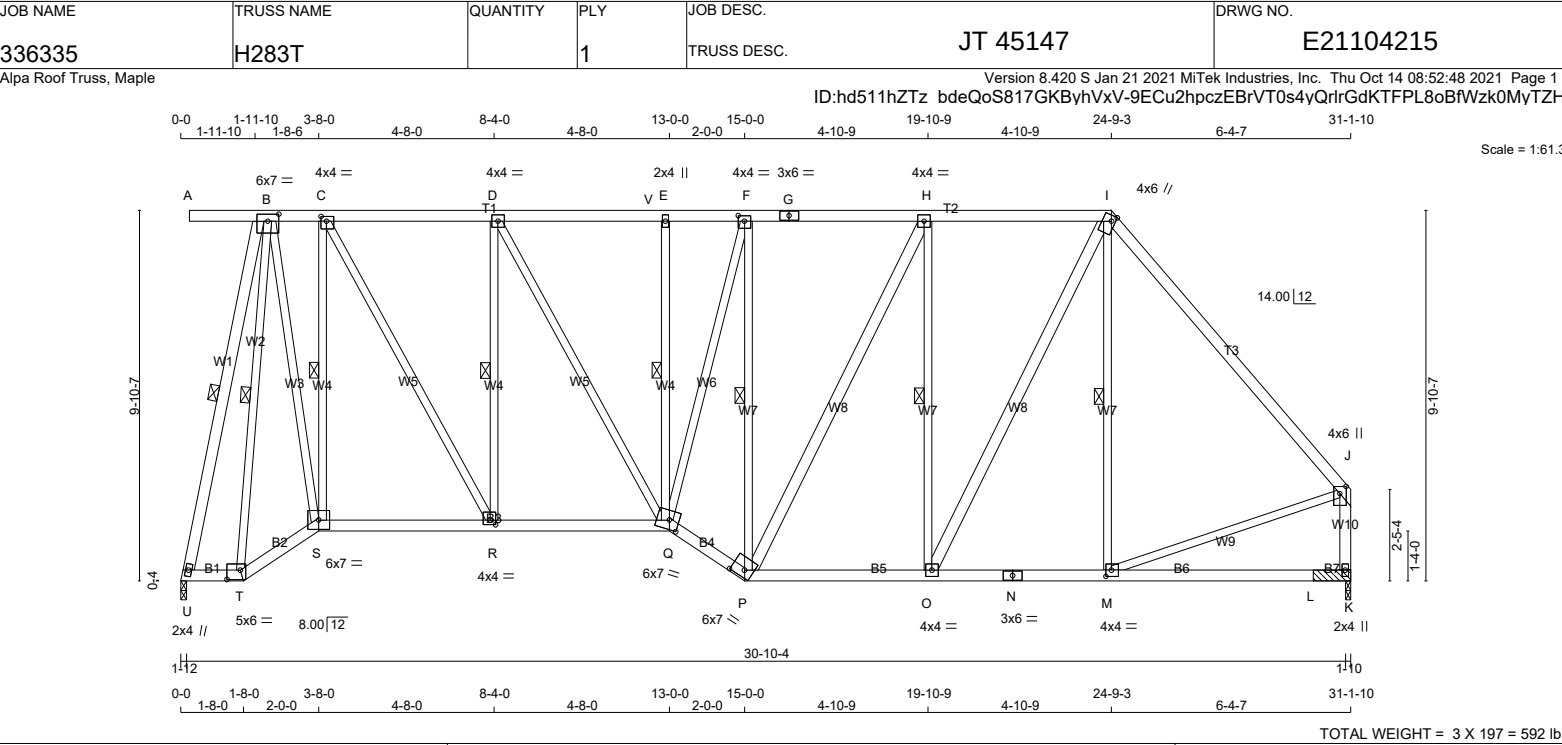
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (Q) (INPUT = 0.90)
JSI METAL= 0.46 (J) (INPUT = 1.00)



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - G	2x4	DRY No.2	SPF
G - I	2x4	DRY No.2	SPF
I - J	2x4	DRY 2100F 1.8E	SPF
K - J	2x4	DRY No.2	SPF
U - T	2x4	DRY No.2	SPF
T - S	2x4	DRY No.2	SPF
S - Q	2x4	DRY No.2	SPF
Q - P	2x4	DRY No.2	SPF
P - N	2x4	DRY No.2	SPF
N - K	2x4	DRY No.2	SPF
ALL WEBS EXCEPT	2x4	DRY No.2	SPF
S - C	2x3	DRY No.2	SPF
C - R	2x3	DRY No.2	SPF
R - D	2x3	DRY No.2	SPF
D - Q	2x3	DRY No.2	SPF
Q - E	2x3	DRY No.2	SPF
Q - F	2x3	DRY No.2	SPF
P - F	2x3	DRY No.2	SPF
O - H	2x3	DRY No.2	SPF
M - I	2x3	DRY No.2	SPF
M - J	2x3	DRY No.2	SPF
T - B	2x3	DRY No.2	SPF
B - S	2x3	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMWWW-t	MT20	6.0	7.0	2.25 3.50
C	TMWW-t	MT20	4.0	4.0	1.50 1.75
D	TMWW-t	MT20	4.0	4.0	
E	TMWW-t	MT20	2.0	4.0	
F	TMWW-t	MT20	4.0	4.0	1.75 2.00
G	TS-t	MT20	3.0	6.0	
H	TMWW-t	MT20	4.0	4.0	
I	TTWW+m	MT20	4.0	6.0	1.75 1.25
J	TMVW+p	MT20	4.0	6.0	2.25 2.00
K	BMV1+p	MT20	2.0	4.0	
M	BMWW-t	MT20	4.0	4.0	2.00 1.75
N	BS-t	MT20	3.0	6.0	
O	BMWW-t	MT20	4.0	4.0	
P	BBWW-h	MT20	6.0	7.0	2.25 4.25

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
U	1601	0	1601	0	0	1-12	1-12
K	1565	0	1565	0	0	1-10	1-10 & BLOCK

UNFACTORED REACTIONS		1ST LCASE		MAX./MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND
U	1151	648 / 0	0 / 0	0 / 0	0 / 0
K	1122	649 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, K

2x4 DRY SPF No.2 BEARING BLOCK 12" LONG AT JT. K ATTACHED TO FRONT SIDE WITH 2 ROWS OF (0.122"x3") SPIRAL NAILS SPACED 3" C.C. 8 NAILS TOTAL.

BRACING

FOR SECTION A-I, MAX. PURLIN SPACING = 2.00 FT.

FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.12 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF B-U, C-S, D-R, E-Q, F-P, H-O, I-M, B-T.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC1)
FR-TO		FROM TO		FR-TO			
A-B	0 / 0	-85.5 -85.5	0.26 (1)	10.00	U-B	-1626 / 0	0.81 (1)
B-C	-603 / 0	-85.5 -85.5	0.26 (1)	2.00	S-C	-1148 / 0	0.53 (1)
C-D	-1192 / 0	-85.5 -85.5	0.29 (1)	2.00	C-R	0 / 1187	0.27 (1)
D-V	-1468 / 0	-85.5 -85.5	0.30 (1)	2.00	R-D	-940 / 0	0.44 (1)
V-E	-1468 / 0	-85.5 -85.5	0.30 (1)	2.00	D-Q	0 / 559	0.13 (1)
E-F	-1465 / 0	-85.5 -85.5	0.16 (1)	2.00	Q-E	-245 / 0	0.11 (1)
F-G	-1302 / 0	-85.5 -85.5	0.34 (1)	2.00	Q-F	0 / 698	0.16 (1)
G-H	-1302 / 0	-85.5 -85.5	0.34 (1)	2.00	P-F	-965 / 0	0.63 (1)
H-I	-1218 / 0	-85.5 -85.5	0.34 (1)	2.00	P-H	0 / 184	0.03 (1)
I-J	-1309 / 0	-78.0 -78.0	0.46 (1)	6.12	O-H	-639 / 0	0.42 (1)
K-J	-1516 / 0	0.0 0.0	0.20 (1)	6.67	O-I	0 / 813	0.13 (1)
					M-I	-149 / 18	0.10 (1)
U-T	0 / 352	-18.5 -18.5	0.06 (1)	10.00	M-J	0 / 892	0.20 (1)
T-S	0 / 440	-18.5 -18.5	0.08 (1)	10.00	T-B	-211 / 0	0.14 (1)
S-R	0 / 617	-18.5 -18.5	0.18 (4)	10.00	B-S	0 / 1464	0.33 (1)
R-Q	0 / 1192	-18.5 -18.5	0.25 (1)	10.00			
Q-P	0 / 1552	-18.5 -18.5	0.26 (1)	10.00			
P-O	0 / 1218	-18.5 -18.5	0.25 (1)	10.00			
O-N	0 / 848	-18.5 -18.5	0.25 (4)	10.00			
N-M	0 / 848	-18.5 -18.5	0.25 (4)	10.00			
M-L	0 / 0	-18.5 -18.5	0.16 (4)	10.00			
L-K	0 / 0	-18.5 -18.5	0.16 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 21.0 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE HEEL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")

CALCULATED VERT. DEFL.(LL) = L/999 (0.08")

ALLOWABLE DEFL.(TL)= L/360 (1.03")

CALCULATED VERT. DEFL.(TL) = L/999 (0.17")

CSI: TC=0.46/1.00 (I-J:1) , BC=0.26/1.00 (P-Q:1) , WB=0.81/1.00 (B-U:1) , SS=0.20/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES			
PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (I) (INPUT= 0.90)

JSI METAL= 0.50 (J) (INPUT= 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
336335	H283T		1	JT 45147	E21104215(2)

Alpa Roof Truss, Maple

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Oct 14 08:52:49 2021 Page 2

ID:hd511hZTz_bdeQoS817GKByhVxV-dQIHG1qFkYJi7db3dgy4H2po4tbe4b2KuAilYoyTZHS

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
Q	BBWWW-m	MT20	6.0	7.0	3.00	3.00
R	BMWW-t	MT20	4.0	4.0	1.50	1.75
S	BBWW-l	MT20	6.0	7.0		
T	BBW-l	MT20	5.0	6.0	3.00	4.25
U	BMW1+w	MT20	2.0	4.0		

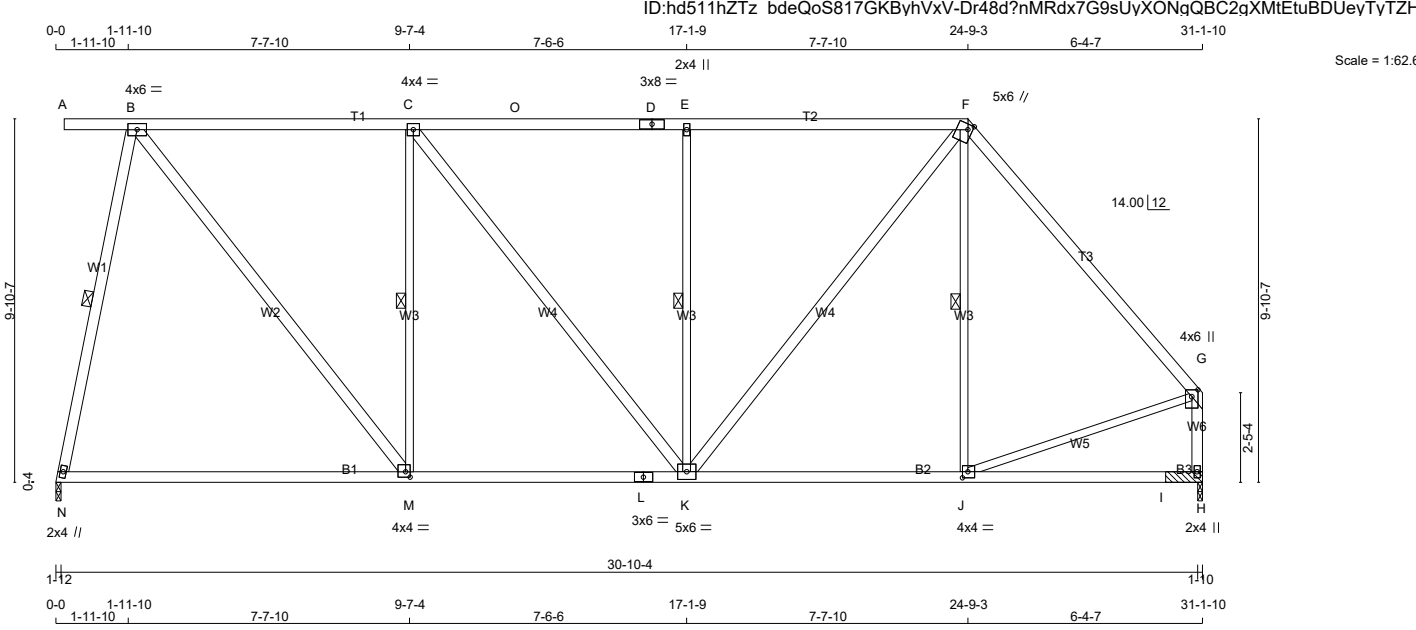
LATERAL BRACE(S) SHOWN SHALL BE
2X4 SPF#2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
336335	H283		1	TRUSS DESC. JT 45147	E21104216

Alpa Roof Truss, Maple

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Oct 14 08:52:46 2021 Page 1



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
D - F	2x4	DRY No.2	SPF
F - G	2x4	DRY 2100F 1.8E	SPF
H - G	2x4	DRY No.2	SPF
N - L	2x4	DRY No.2	SPF
L - H	2x4	DRY No.2	SPF
ALL WEBS	2x4	DRY No.2	SPF
EXCEPT			
M - C	2x3	DRY No.2	SPF
K - E	2x3	DRY No.2	SPF
J - F	2x3	DRY No.2	SPF
J - G	2x3	DRY No.2	SPF
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMWW-t	MT20	4.0	6.0	
C	TMWW-t	MT20	4.0	4.0	
D	TS-t	MT20	3.0	8.0	
E	TMW+w	MT20	2.0	4.0	
F	TTWW+m	MT20	5.0	6.0	1.75 1.50
G	TMVW+p	MT20	4.0	6.0	2.25 2.00
H	BMV1+p	MT20	2.0	4.0	
J	BMWW-t	MT20	4.0	4.0	2.00 1.75
K	BMWWWW-t	MT20	5.0	6.0	
L	BS-t	MT20	3.0	6.0	
M	BMWW-t	MT20	4.0	4.0	1.75 1.50
N	BMW1+w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

		BEARINGS							
		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
		GROSS REACTION		GROSS REACTION		BRG		BRG	
JT	VERT	HORZ		DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
N	1596	0		1596	0	0	1-12	1-12	
H	1570	0		1570	0	0	1-10	1-10 & BLOCK	

		UNFACTORED REACTIONS							
		1ST LCASE		MAX./MIN.		COMPONENT REACTIONS			
JT	COMBINED	SNOW	LIVE	PERM.	LIVE	WIND	DEAD	SOIL	
N	1148	646 / 0	0 / 0	0 / 0	0 / 0	0 / 0	502 / 0	0 / 0	
H	1126	652 / 0	0 / 0	0 / 0	0 / 0	0 / 0	474 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

2x4 DRY SPF No.2 BEARING BLOCK 12" LONG AT JT. H ATTACHED TO FRONT SIDE WITH 2 ROWS OF (0.122"x3") SPIRAL NAILS SPACED 3" C.C. 8 NAILS TOTAL.

BRACING

FOR SECTION A-F, MAX. PURLIN SPACING = 2.00 FT.

FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.10 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF B-N, C-M, E-K, F-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM	TO	FR-TO				
A-B	0 / 0	-85.5	-85.5	0.20 (1)	10.00	N-B	-1555 / 0	0.78 (1)
B-C	-1170 / 0	-85.5	-85.5	0.78 (1)	2.00	B-M	0 / 1409	0.23 (1)
C-O	-1342 / 0	-85.5	-85.5	0.82 (1)	2.00	M-C	-923 / 0	0.61 (1)
O-D	-1342 / 0	-85.5	-85.5	0.82 (1)	2.00	C-K	0 / 278	0.04 (1)
D-E	-1342 / 0	-85.5	-85.5	0.82 (1)	2.00	K-E	-706 / 0	0.46 (1)
E-F	-1342 / 0	-85.5	-85.5	0.83 (1)	2.00	K-F	0 / 781	0.13 (1)
F-G	-1319 / 0	-78.0	-78.0	0.46 (1)	6.10	J-F	-124 / 47	0.08 (1)
H-G	-1525 / 0	0.0	0.0	0.20 (1)	6.65	J-G	0 / 898	0.20 (1)
N-M	0 / 300	-18.5	-18.5	0.39 (4)	10.00			
M-L	0 / 1170	-18.5	-18.5	0.49 (4)	10.00			
L-K	0 / 1170	-18.5	-18.5	0.49 (4)	10.00			
K-J	0 / 854	-18.5	-18.5	0.32 (4)	10.00			
J-I	0 / 0	-18.5	-18.5	0.23 (4)	10.00			
I-H	0 / 0	-18.5	-18.5	0.23 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 21.0 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE HEEL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")

CALCULATED VERT. DEFL. (LL) = L/ 999 (0.05")

ALLOWABLE DEFL.(TL)= L/360 (1.03")

CALCULATED VERT. DEFL. (TL) = L/ 999 (0.23")

CSI: TC=0.83/1.00 (E-F:1) , BC=0.49/1.00 (K-M:4) , WB=0.78/1.00 (B-N:1) , SSI=0.30/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

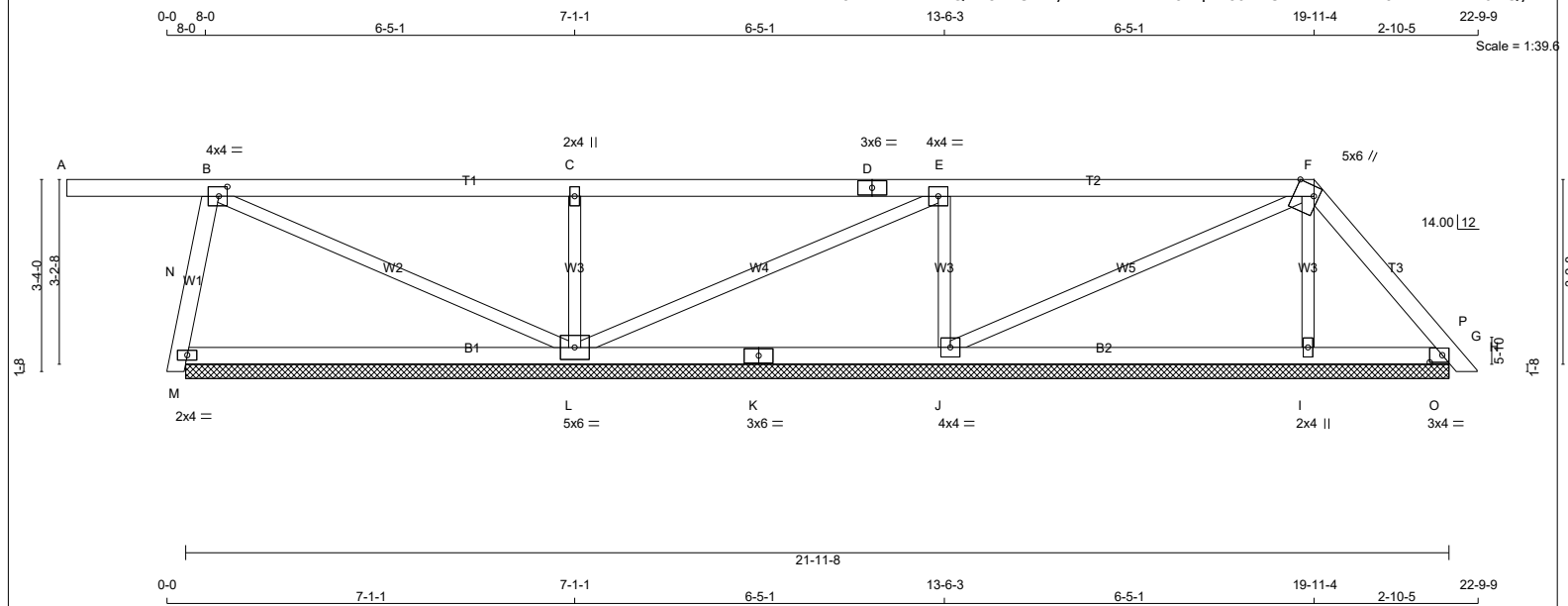
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (M) (INPUT = 0.90)

JSI METAL= 0.50 (G) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2





TOTAL WEIGHT = 2 X 81 = 162 lb

LUMBER

N. L. G. A. RULES

CHORDS		SIZE	LUMBER	DESCR
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
B - L	2x3	DRY	No.2	SPF
L - C	2x3	DRY	No.2	SPF
L - E	2x3	DRY	No.2	SPF
J - E	2x3	DRY	No.2	SPF
J - F	2x3	DRY	No.2	SPF
I - F	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

J	T	TYPE	PLATES	W	LEN	Y	X
B	T	TMWV-t	MT20	4.0	4.0	2.00	1.75
C	D	TMW+w	MT20	2.0	4.0		
B	C	TS-t	MT20	3.0	6.0		
E	F	TMWV-t	MT20	4.0	4.0		
F	F	TTWV+m	MT20	5.0	6.0	Edge	4.00
G	I	TMB1-l	MT20	3.0	4.0	1.50	2.50
I	J	BMV1+w	MT20	2.0	4.0		
J	K	BMWV1-t	MT20	4.0	4.0		
K	L	BS-t	MT20	3.0	6.0		
L	M	BMWVW1-t	MT20	5.0	6.0		
M		BWM1-l	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQ'D BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
JT	407	0	407	0	0	21-11-8	(12-0-0)
M	742	0	742	0	0	21-11-8	(12-0-0)
L	658	0	658	0	0	21-11-8	(12-0-0)
I	301	0	301	0	0	21-11-8	(12-0-0)
G	205	0	205	0	0	21-11-8	(12-0-0)

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	287	192 / 0	0 / 0	0 / 0	0 / 0	95 / 0	0 / 0
L	529	324 / 0	0 / 0	0 / 0	0 / 0	205 / 0	0 / 0
J	468	292 / 0	0 / 0	0 / 0	0 / 0	175 / 0	0 / 0
I	219	107 / 0	0 / 0	0 / 0	0 / 0	113 / 0	0 / 0
G	142	110 / 0	0 / 0	0 / 0	0 / 0	31 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, L, J, I, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 7.81 FT

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEB S				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LOCLC1 (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO			FR-TO			
A-B	0/0	-78.0	-78.0	0.34 (1)	10.00	N-M	0/0	0.00 (1)
B-C	0/9	-78.0	-78.0	0.43 (1)	10.00	M-B	-363/0	0.05 (1)
C-D	0/9	-78.0	-78.0	0.44 (1)	10.00	B-L	-88/0	0.08 (1)
D-E	0/9	-78.0	-78.0	0.44 (1)	10.00	L-C	-544/0	0.10 (1)
E-F	-35/0	-78.0	-78.0	0.44 (1)	6.25	L-E	-48/0	0.05 (1)
F-P	-113/0	-78.0	-78.0	0.05 (1)	6.25	J-E	-533/0	0.10 (1)
P-G	-1/23	-78.0	-78.0	0.04 (1)	10.00	J-F	-28/0	0.03 (1)
G-H	0/10	-78.0	-78.0	0.01 (1)	10.00	I-F	-182/0	0.03 (1)
						O-P	-220/0	0.00 (1)
M-L	0/71	-18.5	-18.5	0.21 (4)	10.00			
L-K	0/35	-18.5	-18.5	0.21 (4)	10.00			
K-J	0/35	-18.5	-18.5	0.21 (4)	10.00			
J-I	0/60	-18.5	-18.5	0.14 (4)	10.00			
I-O	0/67	-18.5	-18.5	0.12 (4)	10.00			
O-G	0/67	-18.5	-18.5	0.06 (1)	10.00			

LATERAL BRACE(S) SHOWN SHALL BE
2X4 SPF#2



DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL	=	21.0	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.4	PSF
TOTAL LOAD				=	34.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.44/1.00 (C-E:1), BC=0.21/1.00 (L-M:4),
WB=0.10/1.00 (C-L:1), SSI=0.23/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

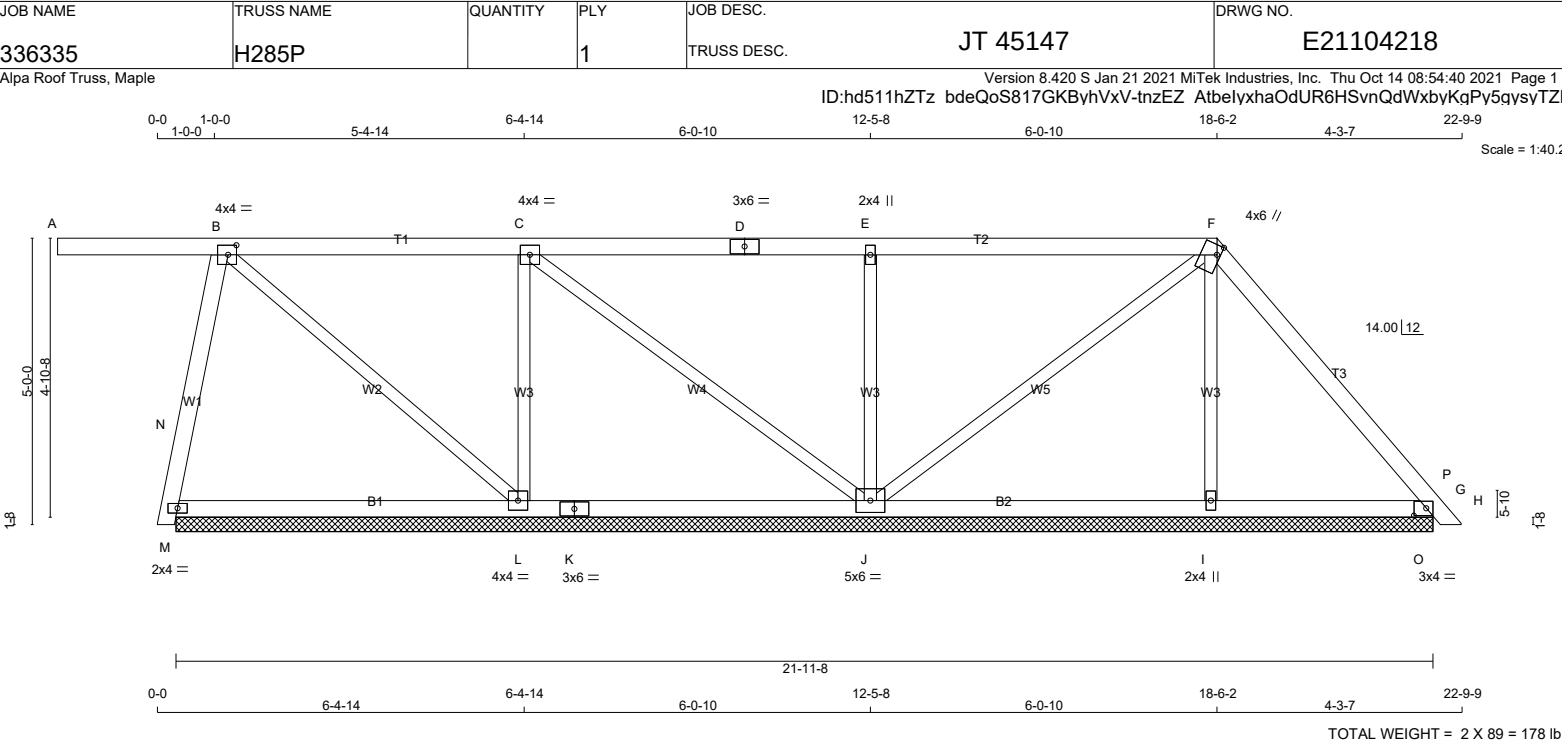
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES						
PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (L) (INPUT = 0.90)
JSI METAL= 0.27 (M) (INPUT = 1.00)



LUMBER				N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF			
D - F	2x4	DRY	No.2	SPF			
F - H	2x4	DRY	No.2	SPF			
M - K	2x4	DRY	No.2	SPF			
K - G	2x4	DRY	No.2	SPF			
ALL WEBS	2x4	DRY	No.2	SPF			
EXCEPT							
B - L	2x3	DRY	No.2	SPF			
L - C	2x3	DRY	No.2	SPF			
C - J	2x3	DRY	No.2	SPF			
J - E	2x3	DRY	No.2	SPF			
J - F	2x3	DRY	No.2	SPF			
I - F	2x3	DRY	No.2	SPF			
DRY: SEASONED LUMBER.							

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	4.0	4.0	2.00	1.75
C	TMWW-t	MT20	4.0	4.0		
D	TS-t	MT20	3.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	4.0	6.0	2.00	0.75
G	TMB1-l	MT20	3.0	4.0	1.50	2.50
I	BMW1+w	MT20	2.0	4.0		
J	BMWWW1-t	MT20	5.0	6.0		
K	BS-t	MT20	3.0	6.0		
L	BMWW1-t	MT20	4.0	4.0		
M	BWM1-l	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
M	397	0	397	0	0	21-11-8 (15-0-0)			
L	597	0	597	0	0	21-11-8 (15-0-0)			
J	748	0	748	0	0	21-11-8 (15-0-0)			
I	283	0	283	0	0	21-11-8 (15-0-0)			
G	287	0	287	0	0	21-11-8 (15-0-0)			

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS		1ST LCASE		MAX./MIN. COMPONENT REACTIONS			
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	280	189 / 0	0 / 0	0 / 0	0 / 0	91 / 0	0 / 0
L	427	252 / 0	0 / 0	0 / 0	0 / 0	175 / 0	0 / 0
J	530	344 / 0	0 / 0	0 / 0	0 / 0	186 / 0	0 / 0
I	207	100 / 0	0 / 0	0 / 0	0 / 0	107 / 0	0 / 0
G	202	140 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, L, J, I, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 7.81 FT

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED FORCE (LBS)		FACTORED VERT. LOAD (PLF)		MAX. FACTORED CS1 (LC)		UNBRACED LENGTH		WEBS		MAX. FACTORED FORCE (LBS)		MAX. FACTORED CS1 (LC)	
MEMB.	FR-TO	FROM	TO	FROM	TO	FROM	TO	FROM	TO	MEMB.	FR-TO	FROM	TO	FROM	TO
A-B	0 / 0	-78.0	-78.0	0.43 (1)	10.00	N-M	0 / 0	0.00 (1)							
B-C	-33 / 0	-78.0	-78.0	0.32 (1)	6.25	M-B	-359 / 0	0.11 (1)							
C-D	0 / 23	-78.0	-78.0	0.41 (1)	10.00	B-L	-49 / 0	0.04 (1)							
D-E	0 / 23	-78.0	-78.0	0.41 (1)	10.00	L-C	-435 / 0	0.15 (1)							
E-F	0 / 24	-78.0	-78.0	0.41 (1)	10.00	C-J	-71 / 0	0.08 (1)							
F-P	-128 / 0	-78.0	-78.0	0.13 (1)	6.25	J-E	-531 / 0	0.19 (1)							
P-G	0 / 158	-78.0	-78.0	0.11 (1)	10.00	J-F	-118 / 0	0.13 (1)							
G-H	0 / 10	-78.0	-78.0	0.01 (1)	10.00	I-F	-152 / 0	0.05 (1)							
						O-P	-477 / 0	0.00 (1)							
M-L	0 / 70	-18.5	-18.5	0.18 (4)	10.00										
L-K	0 / 33	-18.5	-18.5	0.18 (4)	10.00										
K-J	0 / 33	-18.5	-18.5	0.18 (4)	10.00										
J-I	0 / 71	-18.5	-18.5	0.13 (4)	10.00										
I-O	0 / 74	-18.5	-18.5	0.12 (1)	10.00										
O-G	0 / 74	-18.5	-18.5	0.12 (1)	10.00										

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.43/1.00 (A-B:1) , BC=0.18/1.00 (L-M:4) , WB=0.19/1.00 (E-J:1) , SSI=0.37/1.00 (G-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

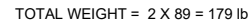
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (F) (INPUT = 0.90)
JSI METAL= 0.27 (M) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Oct 14 08:54:42 2021 Page 1



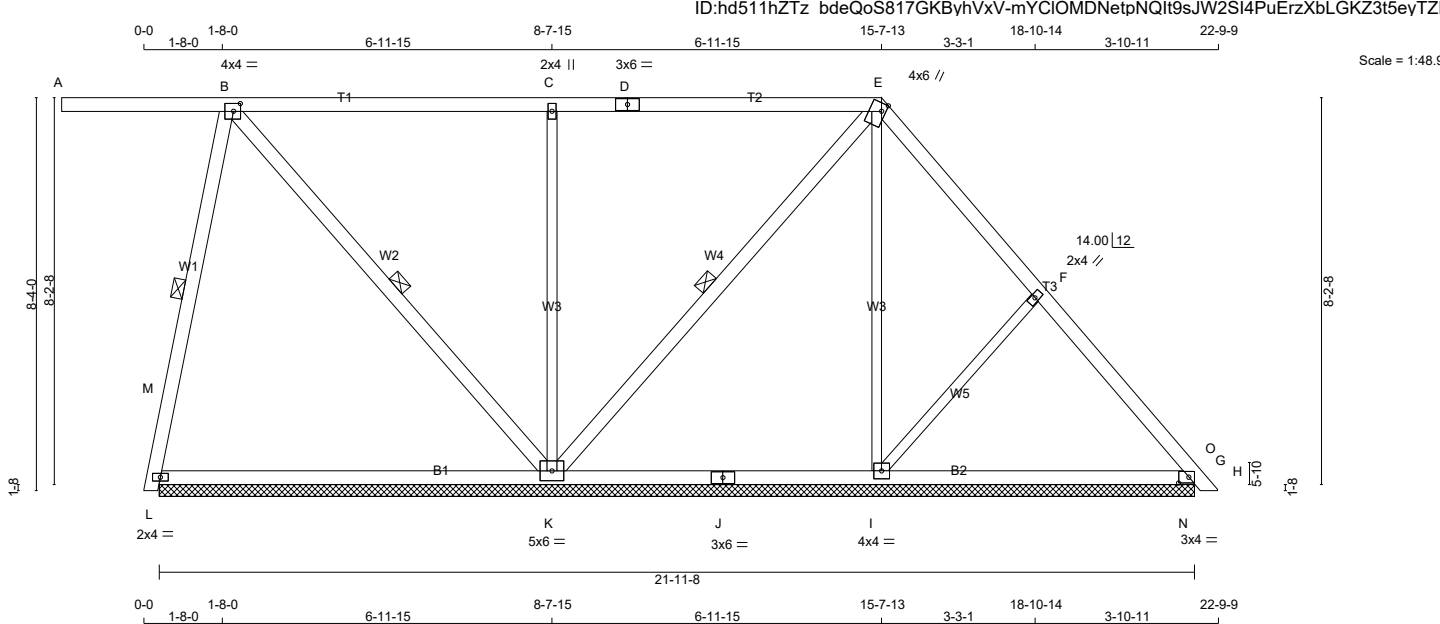
JSI GRIP= 0.89 (E) (INPUT = 0.90)
JSI METAL= 0.33 (K) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
336335	H287P		1	TRUSS DESC. JT 45147	E21104220

Alpa Roof Truss, Maple

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Oct 14 08:54:44 2021 Page 1

ID:hd511hZTz bdeQoS817GKBvVxV-mYCIOMDNetpNQIt9sJW2SI4PuErzXbLGKZ3t5eyTZFF



TOTAL WEIGHT = 2 X 106 = 213 lb [M]

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2 SPF
D - E	2x4	DRY	No.2 SPF
E - H	2x4	DRY	No.2 SPF
L - J	2x4	DRY	No.2 SPF
J - G	2x4	DRY	No.2 SPF
ALL WEBS EXCEPT	2x4	DRY	No.2 SPF
K - C	2x3	DRY	No.2 SPF
I - E	2x3	DRY	No.2 SPF
I - F	2x3	DRY	No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMWW-t	MT20	4.0	4.0	2.00 1.75
C	TMW+w	MT20	2.0	4.0	
D	TS-t	MT20	3.0	6.0	
E	TTWW+m	MT20	4.0	6.0	2.00 1.00
F	TMW+w	MT20	2.0	4.0	
G	TMB1-l	MT20	3.0	4.0	1.50 2.50
I	BMWW1-t	MT20	4.0	4.0	
J	BS-t	MT20	3.0	6.0	
K	BMWWW1-t	MT20	5.0	6.0	
L	BWM1-l	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS					
		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
L	410	0	410	0	0
K	1053	0	1053	0	0
I	464	0	464	0	0
G	386	0	386	0	0

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS							
JT	1ST LCASE	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
L	291	184 / 0	0 / 0	0 / 0	0 / 0	107 / 0	0 / 0
K	747	479 / 0	0 / 0	0 / 0	0 / 0	267 / 0	0 / 0
I	334	187 / 0	0 / 0	0 / 0	0 / 0	147 / 0	0 / 0
G	273	175 / 0	0 / 0	0 / 0	0 / 0	98 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, K, I, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 6.25 FT

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF B-L, B-K, E-K.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 0	-78.0 -78.0	0.66 (1)	M-L	0 / 0	0.00 (1)	
B-C	0 / 42	-78.0 -78.0	0.64 (1)	L-B	-354 / 0	0.12 (1)	
C-D	0 / 41	-78.0 -78.0	0.64 (1)	B-K	-170 / 0	0.10 (1)	
D-E	0 / 41	-78.0 -78.0	0.64 (1)	K-C	-675 / 0	0.89 (1)	
E-F	-79 / 0	-78.0 -78.0	0.15 (1)	K-E	-113 / 0	0.07 (1)	
F-O	-241 / 0	-78.0 -78.0	0.15 (1)	I-E	-178 / 0	0.23 (1)	
O-G	-467 / 0	-78.0 -78.0	0.09 (4)	I-F	-215 / 0	0.10 (1)	
G-H	0 / 10	-78.0 -78.0	0.01 (1)	N-O	0 / 272	0.00 (1)	
L-K	0 / 69	-18.5 -18.5	0.30 (4)				
K-J	0 / 33	-18.5 -18.5	0.30 (4)				
J-I	0 / 33	-18.5 -18.5	0.30 (4)				
I-N	0 / 178	-18.5 -18.5	0.17 (4)				
N-G	0 / 178	-18.5 -18.5	0.09 (4)				

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCS 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.66/1.00 (A-B:1) , BC=0.30/1.00 (K-L:4) , WB=0.89/1.00 (C-K:1) , SSI=0.26/1.00 (C-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

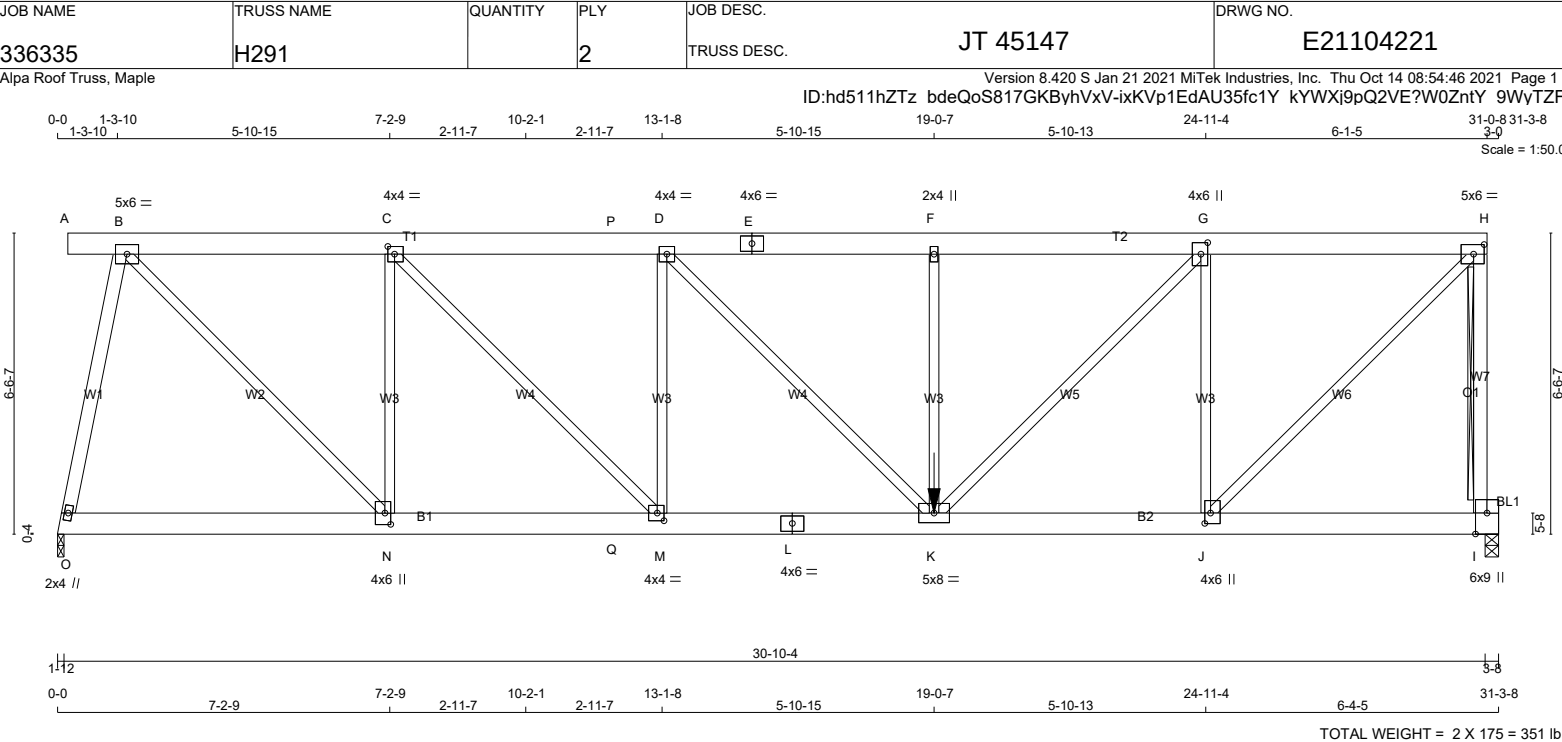
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES					
PLATE	GRIP(DRY)	SHEAR	SECTION		
	(PSI)	(PLI)	(PLI)		
	MAX MIN	MAX MIN	MAX MIN		
MT20	650 371	1747 788	1987 1873		

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.55 (E) (INPUT = 0.90)
JSI METAL= 0.26 (L) (INPUT = 1.00)



1-12 30-10-4 1-3

0-0 7-2-9 7-2-9 2-11-7 10-2-1 2-11-7 13-1-8 5-10-15 19-0-7 5-10-13 24-11-4 6-4-5 31-3-8

TOTAL WEIGHT = 2 X 175 = 351 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - E	2x6	DRY	No.2	SPF
E - H	2x6	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF

BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT				
O - B	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A- E	2 12	SIDE(37.8)
E- H	2 12	TOP
H- I	1 12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
O- L	2 12	SIDE(58.8)
L- I	2 12	SIDE(137.1)
WEBS : (0.122"x3") SPIRAL NAILS		
F- K	1 6	SIDE(229.1)
2x3	1 6	
2x4	1 6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG UPLIFT		REQRD BRG IN-SX	
JT	VERT	HORZ		DOWN	HORZ		IN-SX	IN-SX	
O	3217	0		3217	0	0	1-12	1-12	
I	2944	0		2944	0	0	3-8	2-8	

UNFACTORED REACTIONS		1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
O	2296	1391 / 0	0 / 0	0 / 0	0 / 0	905 / 0	0 / 0		
I	2098	1282 / 0	0 / 0	0 / 0	0 / 0	816 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.20 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT H-I

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
 TOTAL LOAD CASES: (4)

C H O R D S		MAX. FACTORED		FACTORED		W E B S		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX	MAX. UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX	MAX
FR-TO		FROM	TO	CSI (LC)		FR-TO		CSI (LC)	
A- B	0 / 0	-78.0	-78.0	0.02 (1)	10.00	O- B	-3163 / 0	0.75 (1)	
B- C	-3135 / 0	-153.5	-153.5	0.21 (1)	5.99	K- F	-405 / 0	0.13 (1)	
C- P	-4495 / 0	-153.5	-153.5	0.23 (1)	5.20	K- G	0 / 2880	0.36 (1)	
P- D	-4495 / 0	-78.0	-78.0	0.23 (1)	5.20	J- G	-2606 / 0	0.81 (1)	
D- E	-4644 / 0	-78.0	-78.0	0.14 (1)	5.25	J- H	0 / 3821	0.47 (1)	
E- F	-4644 / 0	-78.0	-78.0	0.14 (1)	5.25	B- N	0 / 3672	0.45 (1)	
F- G	-4644 / 0	-78.0	-78.0	0.14 (1)	5.25	N- C	-2419 / 0	0.75 (1)	
G- H	-2639 / 0	-78.0	-78.0	0.11 (1)	6.25	C- M	0 / 1951	0.24 (1)	
I- H	-2891 / 0	0.0	0.0	0.40 (1)	7.81	M- D	-752 / 0	0.23 (1)	
						D- K	0 / 214	0.03 (1)	
O- N	0 / 598	-36.4	-36.4	0.12 (4)	10.00				
N- Q	0 / 3135	-36.4	-36.4	0.29 (1)	10.00				
Q- M	0 / 3135	-136.1	-136.1	0.29 (1)	10.00				
M- L	0 / 4495	-136.1	-136.1	0.44 (1)	10.00				
L- K	0 / 4495	-136.1	-136.1	0.44 (1)	10.00				
K- J	0 / 2639	-18.5	-18.5	0.22 (1)	10.00				
J- I	-69 / 0	-18.5	-18.5	0.04 (4)	6.25				

FACTORED CONCENTRATED LOADS (LBS)		LOC.		MAX-		MAX+		FACE		DIR.		TYPE		HEEL		CONN.	
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.								
K	19-0-7	-1355	-1355	---	FRONT	VERT	TOTAL	---	C1								

CONNECTION REQUIREMENTS

- C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
 GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
 LOADS WERE DERIVED FROM USER INPUT
 NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:			
TOP CH.	LL =	21.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD = 34.4 PSF			

SPACING = **24.0 IN./C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
 SIDE SETBACK = 0-0
 END SETBACK = 5-10-8
 END WALL WIDTH = 0-0
 CORNER FRAMING TYPE: CONVENTIONAL
 END JACK TYPE: CONVENTIONAL
 APPLIED TO FRONT SIDE
 - ADDTL LOADS BASED ON 55 % OF GSL.
 LOADS APPLIED TO FIRST 12-1-0 OF SPAN MEASURED FROM THE LEFT.

GIRDER TYPE: CStdGirder
 START DISTANCE = 12-1-0
 START SPAN CARRIED = 6-10-8
 END DISTANCE = 19-0-7
 END SPAN CARRIED = 6-10-8
 END WALL WIDTH = 0-0
 APPLIED TO FRONT SIDE OF BOTTOM CHORD.
 - ADDTL LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***
 ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF CBC 2018 , ABC 2019
 - PART 9 OF OBC 2012 (2019 AMENDMENT)
 - CSA 086-14
 - TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.02")
 CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
 ALLOWABLE DEFL.(TL)= L/360 (1.02")
 CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.40/1.00 (H-I:1) , BC=0.44/1.00 (K-M:1) , WB=0.81/1.00 (G-J:1) , SSI=0.18/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
336335	H291		2	JT 45147	E21104221(2)

Alpa Roof Truss, Maple

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Oct 14 08:54:46 2021 Page 2

ID:hd511hZTz_bdeQoS817GKByhVxV-ixKVp1EdAU35fc1Y_kYWXj9pQ2VE?W0ZntY_9WyTZFd

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	5.0	6.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TMWW-t	MT20	4.0	4.0		
E	TS-t	MT20	4.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TMWW+t	MT20	4.0	6.0	3.00	1.75
H	TMVW-t	MT20	5.0	6.0	2.50	2.75
I	BMVK1+p	MT20	6.0	9.0	Edge	3.00
J	BMWW+t	MT20	4.0	6.0	2.75	1.50
K	BMWWWW-t	MT20	5.0	8.0		
L	BS-t	MT20	4.0	6.0		
M	BMWW-t	MT20	4.0	4.0	2.00	1.75
N	BMWW+t	MT20	4.0	6.0	3.00	1.50
O	BMW1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX MIN
MT20	650	371	1747 788 1987 1873

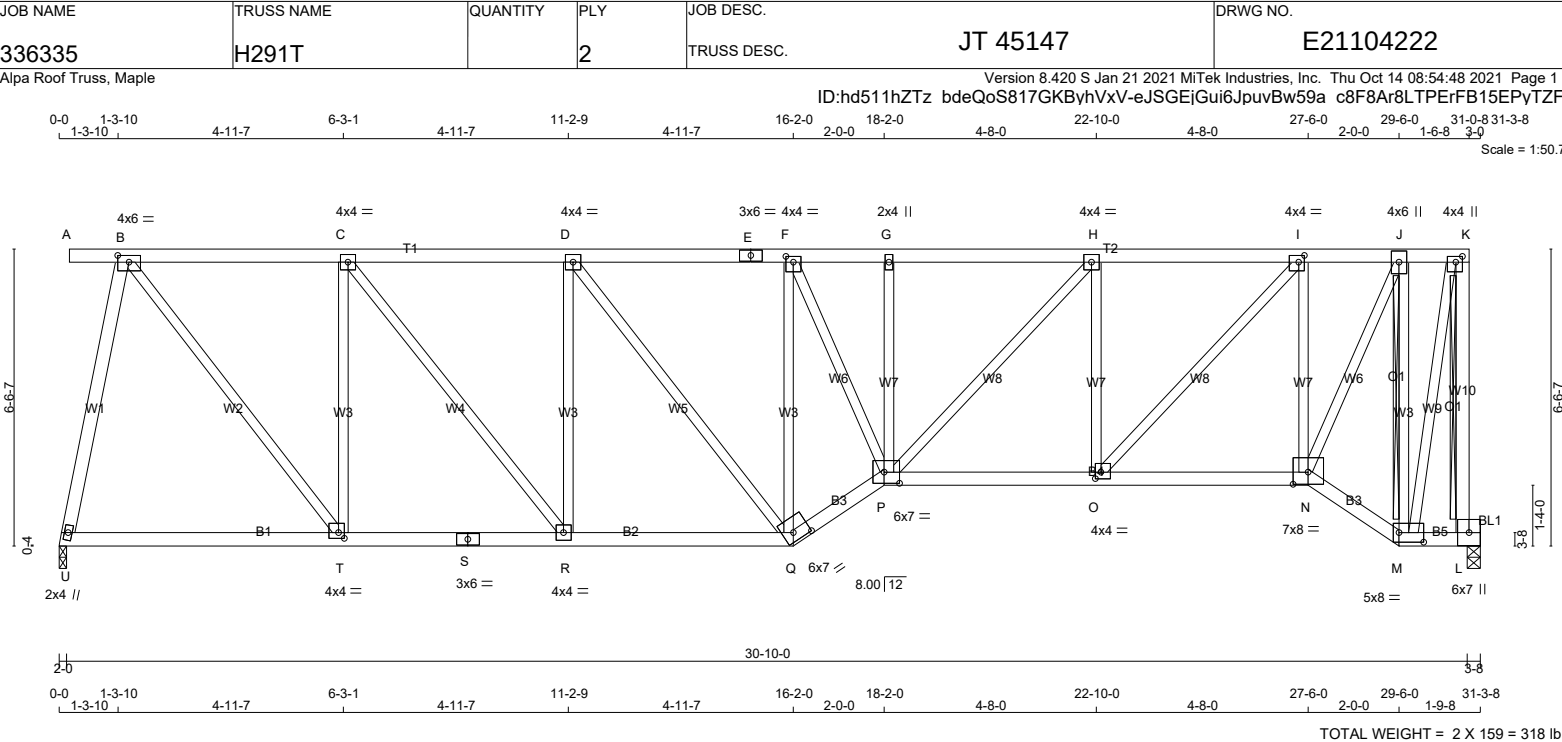
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (N) (INPUT = 0.90)
JSI METAL= 0.77 (L) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2





LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - E	2x4	DRY	No.2 SPF
E - K	2x4	DRY	No.2 SPF
L - K	2x4	DRY	No.2 SPF
U - S	2x4	DRY	No.2 SPF
S - Q	2x4	DRY	No.2 SPF
Q - P	2x4	DRY	No.2 SPF
P - N	2x4	DRY	No.2 SPF
N - M	2x4	DRY	No.2 SPF
M - L	2x4	DRY	No.2 SPF
BEARING BLOCKS			
BL1	2x4	DRY	No.2 SPF
ALL WEBS EXCEPT			
U - B	2x4	DRY	No.2 SPF
DRY: SEASONED LUMBER.			
DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:			
CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)	
TOP CHORDS : (0.122"x3") SPIRAL NAILS			
A - E	12	SIDE(37.8)	
E - K	12	SIDE(37.8)	
K - L	12	TOP	
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS			
U - S	12	SIDE(10.0)	
S - Q	12	SIDE(10.0)	
Q - P	12	SIDE(38.9)	
P - N	12	SIDE(46.7)	
N - M	12	SIDE(38.9)	
M - L	12	SIDE(10.0)	
WEBS : (0.122"x3") SPIRAL NAILS			
2x3	6		
2x4	6		
NAILS TO BE DRIVEN FROM ONE SIDE ONLY.			
TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.			



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
	FACTORED		MAXIMUM FACTORED		INPUT		REQRD		
	GROSS REACTION		GROSS REACTION		BRG		BRG		
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX		
U	2816	0	2816	0	2-0	1-8			
L	2925	0	2925	0	0	3-8	2-8		
UNFACTORED REACTIONS									
	1ST LCASE		MAX./MIN.		COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE		PERM.LIVE	WIND	DEAD	SOIL	
U	2010	1215 / 0	0 / 0		0 / 0	0 / 0	795 / 0	0 / 0	
L	2086	1274 / 0	0 / 0		0 / 0	0 / 0	811 / 0	0 / 0	
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L									
BRACING									
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.35 FT.									
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.									
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.									
2x4 DRY SPF No.2 T-BRACE AT K-L, J-M									
FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.									
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW									
LOADING									
TOTAL LOAD CASES: (4)									
C H O R D S									
	MAX. FACTORED		FACTORED		W E B S		MAX. FACTORED		
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRAC	MEMB.	FORCE (LBS)	MAX. UNBRAC		
FR-TO		FROM TO	CSI (LC)	LENGTH	FR-TO				
A- B	0 / 0	-78.0	-78.0	0.04 (1)	10.00	U- B	-2777 / 0	0.76 (1)	
B- C	-2341 / 0	-153.5	-153.5	0.31 (1)	5.48	B- T	0 / 2955	0.37 (1)	
C- D	-3347 / 0	-153.5	-153.5	0.35 (1)	4.73	T- C	-2104 / 0	0.75 (1)	
D- E	-3611 / 0	-153.5	-153.5	0.31 (1)	4.64	C- R	0 / 1618	0.20 (1)	
E- F	-3611 / 0	-153.5	-153.5	0.31 (1)	4.64	R- D	-1097 / 0	0.39 (1)	
F- G	-4425 / 0	-78.0	-78.0	0.23 (1)	4.35	D- Q	0 / 425	0.05 (1)	
G- H	-4428 / 0	-78.0	-78.0	0.21 (1)	4.39	Q- F	-2512 / 0	0.89 (1)	
H- I	-3606 / 0	-78.0	-78.0	0.18 (1)	4.80	F- P	0 / 2168	0.27 (1)	
I- J	-1806 / 0	-78.0	-78.0	0.10 (1)	6.25	P- G	-149 / 0	0.03 (1)	
J- K	-603 / 0	-153.5	-153.5	0.09 (1)	6.25	P- H	0 / 1191	0.15 (1)	
L- K	-2900 / 0	0.0	0.0	0.45 (1)	7.81	O- H	-1263 / 0	0.26 (1)	
						O- I	0 / 2576	0.32 (1)	
U- T	0 / 532	-36.4	-36.4	0.18 (4)	10.00	N- I	-2227 / 0	0.46 (1)	
T- S	0 / 2341	-36.4	-36.4	0.30 (1)	10.00	N- J	0 / 3172	0.39 (1)	
S- R	0 / 2341	-36.4	-36.4	0.30 (1)	10.00	M- J	-2941 / 0	0.56 (1)	
R- Q	0 / 3347	-36.4	-36.4	0.36 (1)	10.00	M- K	0 / 2790	0.35 (1)	
Q- P	0 / 4289	-112.0	-112.0	0.42 (1)	10.00				
P- O	0 / 3606	-112.0	-112.0	0.53 (1)	10.00				
O- N	0 / 1853	-112.0	-112.0	0.37 (1)	10.00				
N- M	0 / 665	-112.0	-112.0	0.10 (1)	10.00				
M- L	-68 / 0	-36.4	-36.4	0.01 (4)	6.25				

DESIGN CRITERIA			
SPECIFIED LOADS:			
TOP CH.	LL	= 21.0	PSF
	DL	= 6.0	PSF
BOT CH.	LL	= 0.0	PSF
	DL	= 7.4	PSF
TOTAL LOAD = 34.4 PSF			
SPACING = 24.0 IN. C/C			
LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM			
GIRDER TYPE: CPrimeHip			
SIDE SETBACK = 0-0			
END SETBACK = 5-10-8			
START SPAN CARRIED = 0-0			
CORNER FRAMING TYPE: CONVENTIONAL			
END JACK TYPE: CONVENTIONAL			
APPLIED TO FRONT SIDE			
- ADDTL LOADS BASED ON 55 % OF GSL.			
LOADS APPLIED TO FIRST 16-2-0 OF SPAN			
MEASURED FROM THE LEFT.			
GIRDER TYPE: CStdGirder			
START DISTANCE = 16-2-0			
START SPAN CARRIED = 5-10-8			
END DISTANCE = 29-6-0			
END SPAN CARRIED = 5-10-8			
END WALL WIDTH = 0-0			
APPLIED TO FRONT SIDE OF BOTTOM CHORD.			
- ADDTL LOADS BASED ON 55 % OF GSL.			
GIRDER TYPE: CPrimeHip			
SIDE SETBACK = 0-0			
END SETBACK = 5-10-8			
END WALL WIDTH = 0-0			
CORNER FRAMING TYPE: CONVENTIONAL			
END JACK TYPE: CONVENTIONAL			
APPLIED TO FRONT SIDE			
- ADDTL LOADS BASED ON 55 % OF GSL.			
LOADS APPLIED TO FIRST 1-9-8 OF SPAN			
MEASURED FROM THE RIGHT.			
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015			
THIS DESIGN COMPLIES WITH:			
- PART 9 OF BCBC 2018 , ABC 2019			
- PART 9 OF OBC 2012 (2019 AMENDMENT)			
- CSA 086-14			
- TPIC 2014			
(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD			
ALLOWABLE DEFL.(LL)= L/360 (1.02")			
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")			
ALLOWABLE DEFL.(TL)= L/360 (1.02")			
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.26")			
CSI: TC=0.45/1.00 (K-L:1), BC=0.53/1.00 (O-P:1), WB=0.89/1.00 (F-Q:1), SS=0.19/1.00 (B-C:1)			
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00			
COMPANION LIVE LOAD FACTOR = 1.00			

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
336335	H291T		2	JT 45147	E21104222(2)

Alpa Roof Truss, Maple

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Oct 14 08:54:48 2021 Page 2

ID:hd511hZTz_bdeQoS817GKByhVxV-eJSGEjGui6JpuvBw59a_c8F8Ar8LTPErFB15EPyTZFb

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	4.0	6.0	1.75	3.00
C, D, H						
C	TMWW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0	1.50	2.00
G	TMW+w	MT20	2.0	4.0		
I	TMWW-t	MT20	4.0	4.0	1.75	1.50
J	TMWW+t	MT20	4.0	6.0		
K	TMVW+p	MT20	4.0	4.0	1.50	1.75
L	BMVK1+p	MT20	6.0	7.0		
M	BBWW-l	MT20	5.0	8.0	2.50	6.50
N	BBWW-l	MT20	7.0	8.0	3.25	4.00
O	BMWW-t	MT20	4.0	4.0	1.75	1.50
P	BBWWW-p	MT20	6.0	7.0	3.00	4.00
Q	BBWW-h	MT20	6.0	7.0	2.25	4.25
R	BMWW-t	MT20	4.0	4.0		
S	BS-t	MT20	3.0	6.0		
T	BMWW-t	MT20	4.0	4.0	1.50	1.50
U	BMW1+w	MT20	2.0	4.0		

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

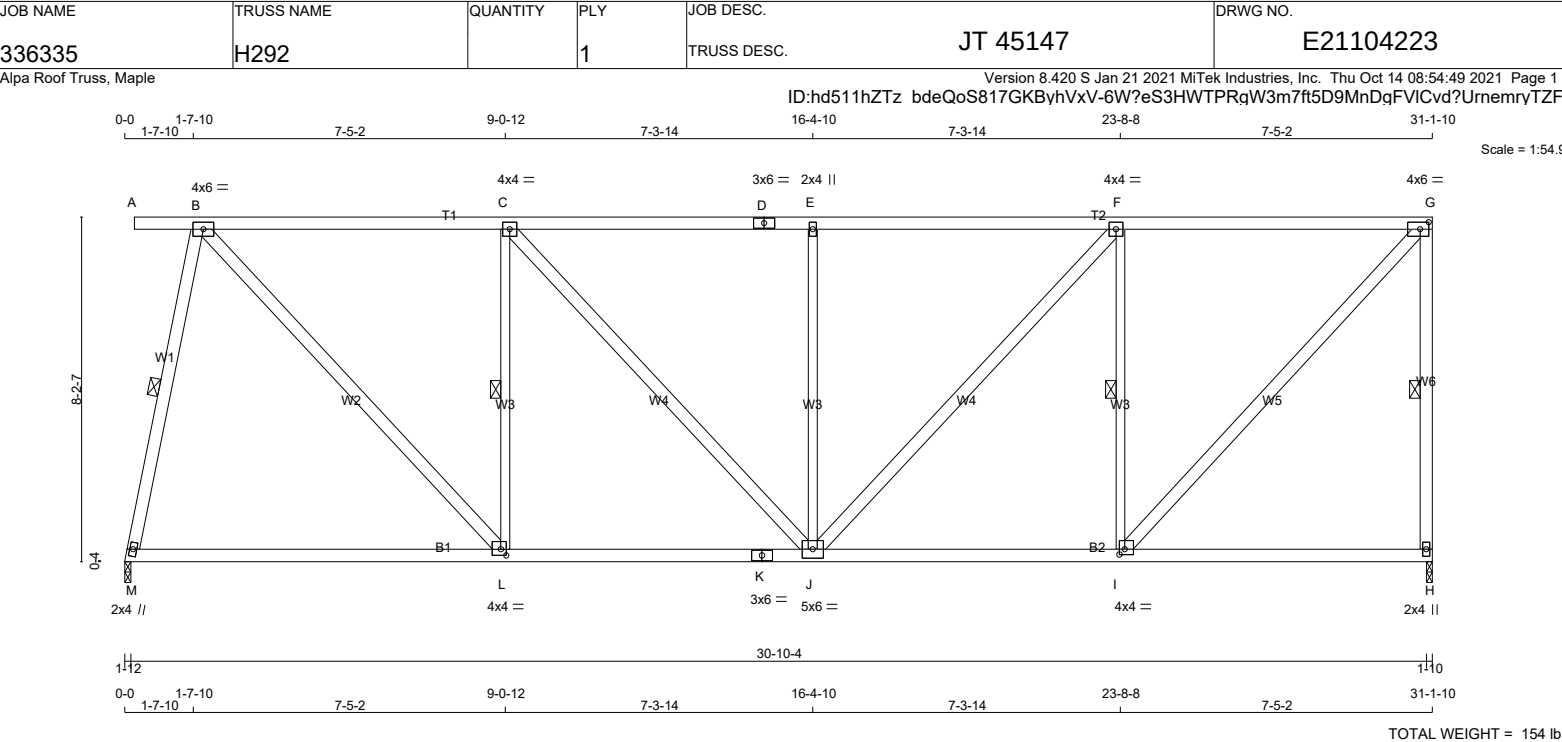
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (J) (INPUT = 0.90)

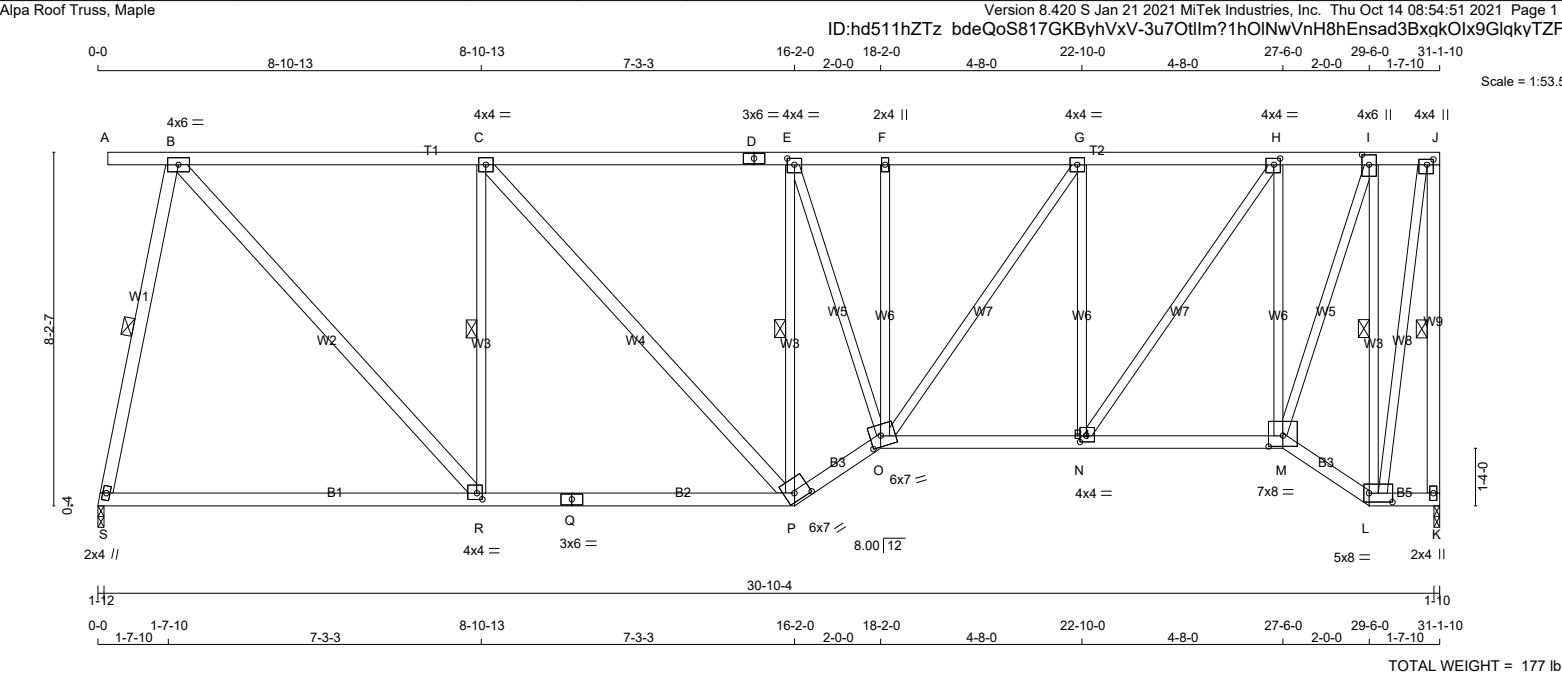
JSI METAL= 0.49 (P) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2





JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
336335	H292T		1	TRUSS DESC. JT 45147	E21104224



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. A - D 2x4 DRY No.2 SPF D - J 2x4 DRY No.2 SPF K - J 2x4 DRY No.2 SPF S - Q 2x4 DRY No.2 SPF Q - P 2x4 DRY No.2 SPF P - O 2x4 DRY No.2 SPF O - M 2x4 DRY No.2 SPF M - L 2x4 DRY No.2 SPF L - K 2x4 DRY No.2 SPF ALL WEBS 2x4 DRY No.2 SPF EXCEPT R - C 2x3 DRY No.2 SPF P - E 2x3 DRY No.2 SPF E - O 2x3 DRY No.2 SPF O - F 2x3 DRY No.2 SPF O - G 2x3 DRY No.2 SPF N - G 2x3 DRY No.2 SPF N - H 2x3 DRY No.2 SPF M - H 2x3 DRY No.2 SPF M - I 2x3 DRY No.2 SPF L - I 2x3 DRY No.2 SPF L - J 2x3 DRY No.2 SPF				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS FACTORED MAXIMUM FACTORED INPUT REQD GROSS REACTION GROSS REACTION BRG JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX S 1487 0 1487 0 0 1-12 1-10 K 1495 0 1495 0 0 1-10 1-10 UNFACTORED REACTIONS 1ST LCASE MAX./MIN. COMPONENT REACTIONS JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL S 1060 647 / 0 0 / 0 0 / 0 0 / 0 414 / 0 0 / 0 K 1066 651 / 0 0 / 0 0 / 0 0 / 0 415 / 0 0 / 0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.33 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. 1 LATERAL BRACE(S) AT 1 / 2 LENGTH OF B-S, J-K, C-R, E-P, I-L. END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW LOADING TOTAL LOAD CASES: (4)				DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 21.0 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.4 PSF TOTAL LOAD = 34.4 PSF SPACING = 24.0 IN. C/C LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF CBC 2018 , ABC 2019 - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014 (55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL)= L/360 (1.03") CALCULATED VERT. DEFL.(LL)= L/ 999 (0.11") ALLOWABLE DEFL.(TL)= L/360 (1.03") CALCULATED VERT. DEFL.(TL)= L/ 999 (0.21") CSI: TC=0.76/1.00 (C-E:1) , BC=0.47/1.00 (P-R:4) , WB=1.00/1.00 (H-M:1) , SSI=0.27/1.00 (B-C:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10 COMPANION LIVE LOAD FACTOR = 1.00 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT . NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.90 (R) (INPUT = 0.90) JSI METAL= 0.37 (Q) (INPUT = 1.00)			
--	--	--	--	---	--	--	--	--	--	--	--

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMWW-t	MT20	4.0	6.0		
C TMWW-t	MT20	4.0	4.0		
D TS-t	MT20	3.0	6.0		
E TMWW-t	MT20	4.0	4.0	1.75	2.00
F TMW+w	MT20	2.0	4.0		
G TMWW-t	MT20	4.0	4.0		
H TMWW-t	MT20	4.0	4.0	1.75	1.75
I TMWW+t	MT20	4.0	6.0	2.75	2.00
J TMVW+p	MT20	4.0	4.0	1.50	1.75
K BMV1+p	MT20	2.0	4.0		
L BBWW-l	MT20	5.0	8.0	2.50	6.50
M BBWW-l	MT20	7.0	8.0	3.00	4.00
N BBWW-t	MT20	4.0	4.0	1.75	1.75
O BBWWW-m	MT20	6.0	7.0	3.00	3.00
P BBWW-h	MT20	6.0	7.0	2.25	4.25
Q BS-t	MT20	3.0	6.0		

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2





TOTAL WEIGHT = 3 X 199 = 596 lb

DRY: SEASONED LUMBER.

LATERAL BRACE(S) SHOWN SHALL BE
2X4 SPF#2

TOTAL LOAD CASES: (4)

JSI GRIP= 0.90 (J) (INPUT = 0.90)
JSI METAL= 0.37 (R) (INPUT = 1.00)

TOTAL WEIGHT = 9 X 169 = 1522 lb

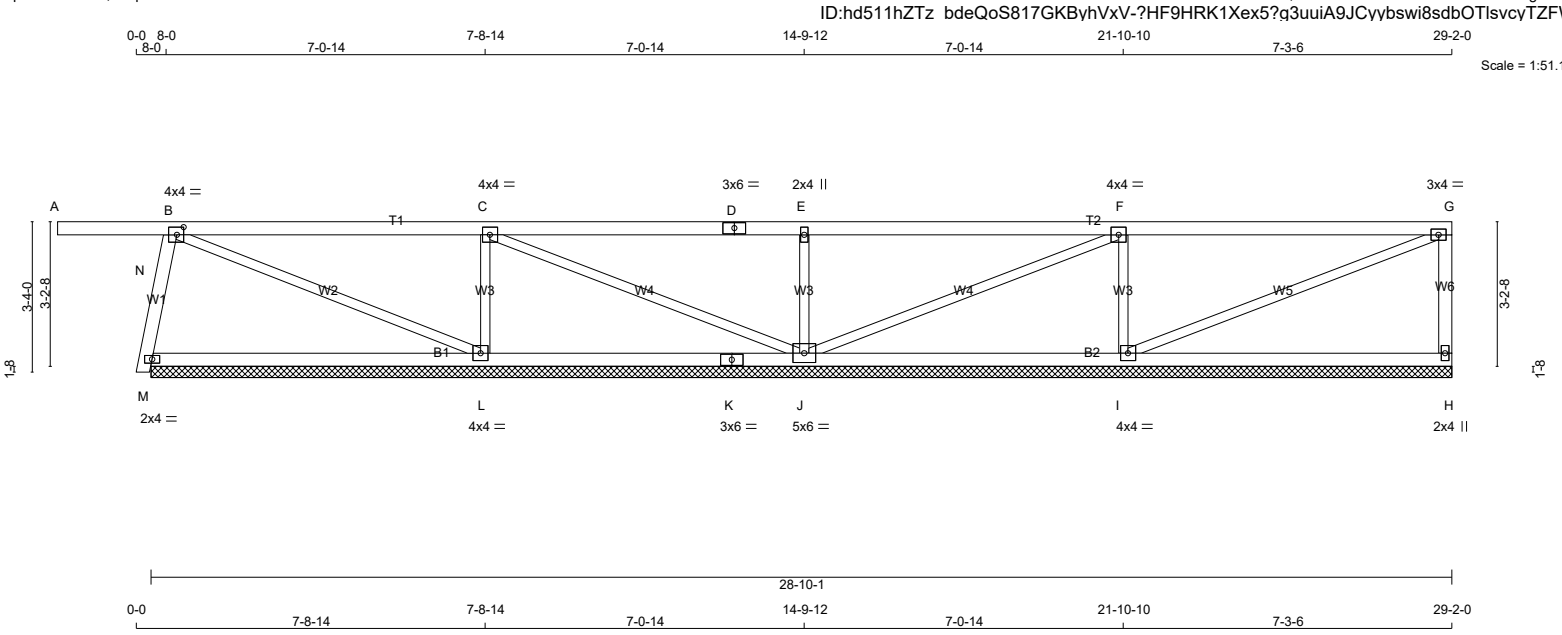
JSI GRIP= 0.89 (J) (INPUT = 0.90)
JSI METAL= 0.37 (J) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
336335	H294P		1	TRUSS DESC. JT 45147	E21104227

Alpa Roof Truss, Maple

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Oct 14 08:54:53 2021 Page 1

ID:hd511hZTz bdeQoS817GKByhVxV-?HF9HRK1Xex5?q3uuiA9JCyybswi8sdbOTlsvcyTZFW



LUMBER

N. L. G. A. RULES

CHORDS SIZE LUMBER DESCR.

A - D 2x4 DRY No.2 SPF

D - G 2x4 DRY No.2 SPF

H - G 2x4 DRY No.2 SPF

M - K 2x4 DRY No.2 SPF

K - H 2x4 DRY No.2 SPF

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

N - B 2x4 DRY No.2 SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
		GROSS REACTION		GROSS REACTION		BRG		BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
H	280	0	280	0	0	28-10-1 (15-11-8)			
M	441	0	441	0	0	28-10-1 (15-11-8)			
L	777	0	777	0	0	28-10-1 (15-11-8)			
J	676	0	676	0	0	28-10-1 (15-11-8)			
I	771	0	771	0	0	28-10-1 (15-11-8)			

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS									
		1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
H	199	122 / 0	0 / 0	0 / 0	0 / 0	77 / 0	0 / 0		
M	311	208 / 0	0 / 0	0 / 0	0 / 0	104 / 0	0 / 0		
L	554	334 / 0	0 / 0	0 / 0	0 / 0	220 / 0	0 / 0		
J	481	300 / 0	0 / 0	0 / 0	0 / 0	181 / 0	0 / 0		
I	550	333 / 0	0 / 0	0 / 0	0 / 0	217 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M, L, J, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 7.81 FT

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

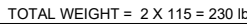
LOADING

TOTAL LOAD CASES: (4)

CHORDS									
		MAX. FACTORED		FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX	MAX. UNBRAC	MEMB.	FORCE (LBS)	MAX	MAX (LC)
FR-TO		FROM	TO	LENGTH	FR-TO				
A-B	0 / 0	-78.0	-78.0	0.34 (1)	10.00	N-M	0 / 0	0.00 (1)	
B-C	-22 / 0	-78.0	-78.0	0.56 (1)	6.25	M-B	-394 / 0	0.06 (1)	
C-D	0 / 43	-78.0	-78.0	0.56 (1)	10.00	B-L	-60 / 0	0.07 (1)	
D-E	0 / 43	-78.0	-78.0	0.56 (1)	10.00	L-C	-597 / 0	0.11 (1)	
E-F	0 / 43	-78.0	-78.0	0.60 (1)	10.00	C-J	-71 / 0	0.08 (1)	
F-G	-6 / 0	-78.0	-78.0	0.59 (1)	10.00	J-E	-511 / 0	0.10 (1)	
H-G	-227 / 0	0.0	0.0	0.04 (1)	7.81	J-F	-53 / 0	0.06 (1)	
						I-F	-621 / 0	0.12 (1)	
						I-G	0 / 6	0.00 (1)	
M-L	0 / 77	-18.5	-18.5	0.26 (4)	10.00				
L-K	0 / 22	-18.5	-18.5	0.26 (4)	10.00				
K-J	0 / 22	-18.5	-18.5	0.26 (4)	10.00				
J-I	0 / 6	-18.5	-18.5	0.24 (4)	10.00				
I-H	0 / 0	-18.5	-18.5	0.24 (4)	10.00				

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



Scale = 1:51.1

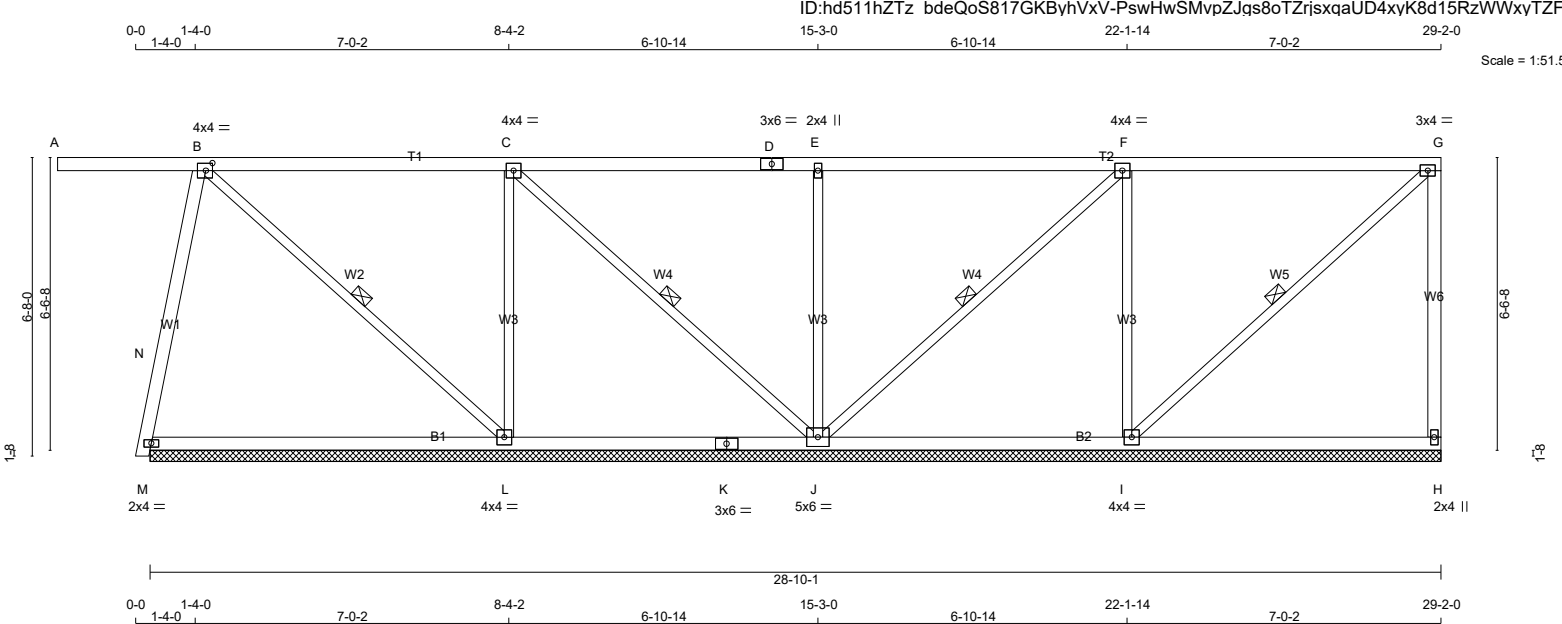
JSI GRIP= 0.76 (G) (INPUT = 0.90)
JSI METAL= 0.30 (D) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
336335	H296P		1	TRUSS DESC. JT 45147	E21104229

Alpa Roof Truss, Maple

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Oct 14 08:54:56 2021 Page 1

ID:hd511hZTz bdeQoS817GKBvhVxV-PswHwSMvpZJgs8oTZrjsxqaUD4xyK8d15RzVWwxyTZFT



TOTAL WEIGHT = 2 X 126 = 252 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
N - B	2x4	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMWW-t	MT20	4.0	4.0	2.00 1.75
C	TMWW-t	MT20	4.0	4.0	
D	TS-t	MT20	3.0	6.0	
E	TMW+w	MT20	2.0	4.0	
F	TMWW-t	MT20	4.0	4.0	
G	TMVW-t	MT20	3.0	4.0	
H	BMV1+p	MT20	2.0	4.0	
I	BMWW1-t	MT20	4.0	4.0	
J	BMWW1-t	MT20	5.0	6.0	
K	BS-t	MT20	3.0	6.0	
L	BMWW1-t	MT20	4.0	4.0	
M	BWM1-t	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
		GROSS REACTION		GROSS REACTION		BRG		BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX		
H	263	0	263	0	0	28-10-1 (15-11-8)			
M	454	0	454	0	0	28-10-1 (15-11-8)			
L	723	0	723	0	0	28-10-1 (15-11-8)			
J	846	0	846	0	0	28-10-1 (15-11-8)			
I	660	0	660	0	0	28-10-1 (15-11-8)			

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS		1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
H	188	114 / 0	0 / 0	0 / 0	0 / 0	73 / 0	0 / 0		
M	321	212 / 0	0 / 0	0 / 0	0 / 0	109 / 0	0 / 0		
L	518	301 / 0	0 / 0	0 / 0	0 / 0	217 / 0	0 / 0		
J	597	395 / 0	0 / 0	0 / 0	0 / 0	203 / 0	0 / 0		
I	473	275 / 0	0 / 0	0 / 0	0 / 0	198 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M, L, J, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 7.81 FT

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF B-L, C-J, F-J, G-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MAX. FACTORED		MAX. FACTORED	
MEMB.	FORCE (LBS)	MEMB.	FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 0	N-M	0 / 0
B-C	-6 / 0	M-B	-402 / 0
C-D	0 / 128	B-L	-98 / 0
D-E	0 / 128	L-C	-493 / 0
E-F	0 / 128	C-J	-181 / 0
F-G	0 / 3	J-E	-499 / 0
H-G	-212 / 0	J-F	-169 / 0
		I-F	-508 / 0
		I-G	-5 / 0
M-L	0 / 79		
L-K	0 / 6		
K-J	0 / 6		
J-I	-3 / 0		
I-H	0 / 0		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.57/1.00 (E-F:1), BC=0.29/1.00 (L-M:4), WB=0.36/1.00 (F-I:1), SSI=0.26/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (G) (INPUT = 0.90)
JSI METAL= 0.30 (M) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

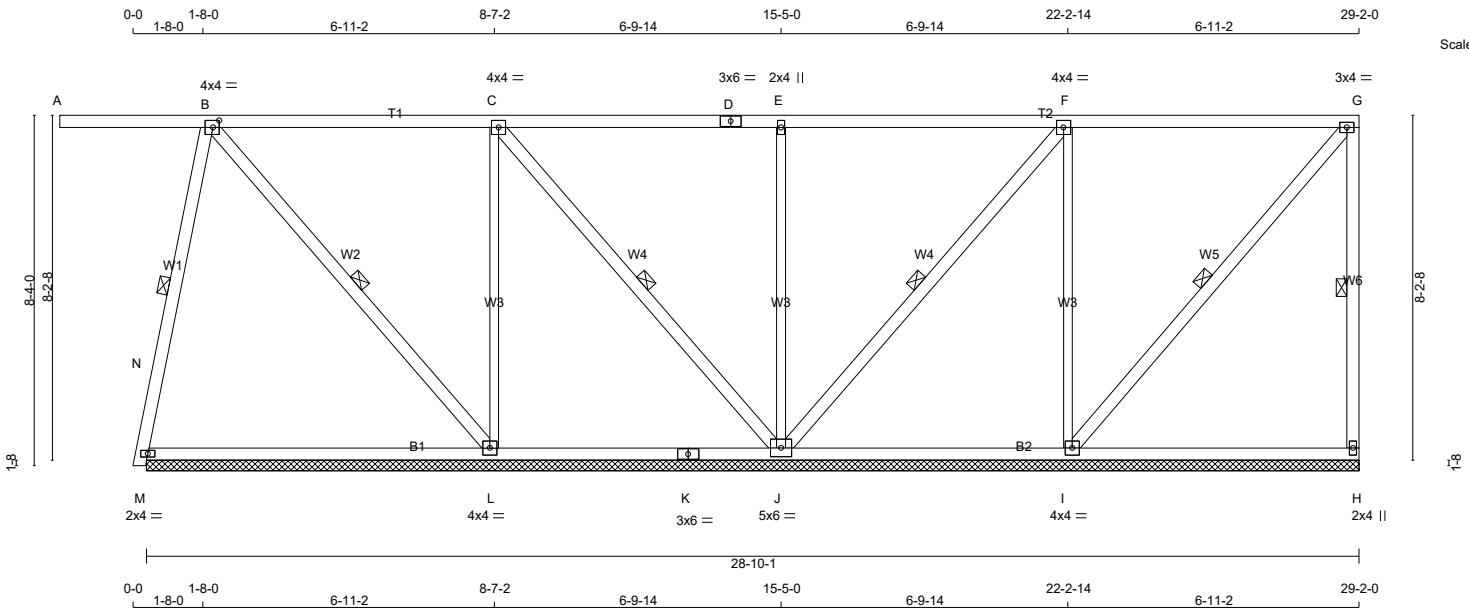


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
336335	H297P		1	TRUSS DESC. JT 45147	E21104230

Alpa Roof Truss, Maple

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Oct 14 08:54:58 2021 Page 1

ID:hd511hZTz bdeQoS817GKByhVxV-LE22L8O9LAaO5SyrhGmK0FfoMtdDozaKYISdaqvTZFR



LUMBER				N. L. G. A. RULES	
CHORDS	SIZE	LUMBER	DESCR.	SPF	SPF
A - D	2x4	DRY	No.2	SPF	SPF
D - G	2x4	DRY	No.2	SPF	SPF
H - G	2x4	DRY	No.2	SPF	SPF
M - K	2x4	DRY	No.2	SPF	SPF
K - H	2x4	DRY	No.2	SPF	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF	SPF
L - C	2x3	DRY	No.2	SPF	SPF
J - E	2x3	DRY	No.2	SPF	SPF
I - F	2x3	DRY	No.2	SPF	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMWW-t	MT20	4.0	4.0	2.00	1.75
C TMWW-t	MT20	4.0	4.0		
D TS-t	MT20	3.0	6.0		
E TMW+w	MT20	2.0	4.0		
F TMWW-t	MT20	4.0	4.0		
G TMVW-t	MT20	3.0	4.0		
H BMV1+p	MT20	2.0	4.0		
I BMWW1-t	MT20	4.0	4.0		
J BMWWW1-t	MT20	5.0	6.0		
K BS-t	MT20	3.0	6.0		
L BMWW1-t	MT20	4.0	4.0		
M BWM1-t	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
H	248	0	248	0	28-10-1 (15-11-8)	
M	443	0	443	0	28-10-1 (15-11-8)	
L	683	0	683	0	28-10-1 (15-11-8)	
J	991	0	991	0	28-10-1 (15-11-8)	
I	582	0	582	0	28-10-1 (15-11-8)	

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	177	106 / 0	0 / 0	0 / 0	0 / 0	71 / 0	0 / 0
M	313	205 / 0	0 / 0	0 / 0	0 / 0	109 / 0	0 / 0
L	491	278 / 0	0 / 0	0 / 0	0 / 0	213 / 0	0 / 0
J	697	475 / 0	0 / 0	0 / 0	0 / 0	223 / 0	0 / 0
I	419	234 / 0	0 / 0	0 / 0	0 / 0	185 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M, L, J, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 6.25 FT

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF B-M, G-H, B-L, C-J, F-J, G-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	CHORDS			MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	WEBS		
		VERT. LOAD (PLF)	LC1	MAX				VERT. LOAD (PLF)	LC1	MAX
FR-TO		FROM	TO	CSI (LC)		FR-TO		FROM	TO	CSI (LC)
A-B	0 / 0	-78.0	-78.0	0.66 (1)	10.00	N-M	0 / 0	0.00	(1)	
B-C	0 / 13	-78.0	-78.0	0.53 (1)	10.00	M-B	-387 / 0	0.13	(1)	
C-D	0 / 181	-78.0	-78.0	0.56 (1)	10.00	B-L	-136 / 0	0.08	(1)	
D-E	0 / 181	-78.0	-78.0	0.56 (1)	10.00	L-C	-413 / 0	0.54	(1)	
E-F	0 / 181	-78.0	-78.0	0.57 (1)	10.00	C-J	-257 / 0	0.15	(1)	
F-G	0 / 13	-78.0	-78.0	0.54 (1)	10.00	J-E	-492 / 0	0.65	(1)	
H-G	-198 / 0	0.0	0.0	0.06 (1)	6.25	J-F	-257 / 0	0.15	(1)	
						I-F	-419 / 0	0.55	(1)	
M-L	0 / 76	-18.5	-18.5	0.30 (4)	10.00	I-G	-20 / 0	0.01	(1)	
L-K	-13 / 0	-18.5	-18.5	0.30 (4)	6.25					
K-J	-13 / 0	-18.5	-18.5	0.30 (4)	6.25					
J-I	-13 / 0	-18.5	-18.5	0.23 (4)	6.25					
I-H	0 / 0	-18.5	-18.5	0.23 (4)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	34.4	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.66/1.00 (A-B:1) , BC=0.30/1.00 (L-M:4) , WB=0.65/1.00 (E-J:1) , SSI=0.26/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	650	371	1747
			788
			1987
			1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.71 (G) (INPUT = 0.90)
JSI METAL= 0.29 (M) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

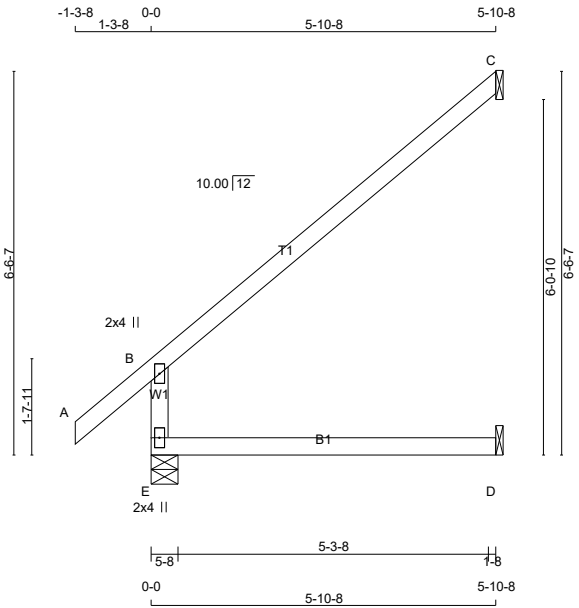


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
336335	J01		1	TRUSS DESC. JT 45147	E21104231

Alpa Roof Truss, Maple

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Oct 14 08:54:59 2021 Page 1

ID:hd511hZTz bdeQoS817GKByhVxV-qRcQYUOn6UiFibX2FzHZZTC0EH?1XawTnPCA6GvTZFQ



Scale = 1:39.3

TOTAL WEIGHT = 28 X 19 = 532 lb

[M]

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER		DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	457	0	457	0	5-8	1-8
C	172	0	172	0	1-8	1-8
D	46	0	52	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	323	212 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0
C	119	93 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0
D	37	0 / 0	0 / 0	0 / 0	0 / 0	37 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS			
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)
FR-TO		FROM	TO			
E-B	-394 / 0	0.0	0.0	0.11 (4)	7.81	
A-B	0 / 35	-78.0	-78.0	0.11 (1)	10.00	
B-C	-37 / 0	-78.0	-78.0	0.46 (1)	6.25	
E-D	0 / 0	-18.5	-18.5	0.14 (4)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:			
TOP CH.	LL	=	21.0 PSF
	DL	=	6.0 PSF
BOT CH.	LL	=	0.0 PSF
	DL	=	7.4 PSF
TOTAL LOAD	=	34.4	PSF

SPACING = 24.0 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.00")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.00")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.46/1.00 (B-C:1) , BC=0.14/1.00 (D-E:4) ,
WB=0.00/1.00 (n/a:0) , SSI=0.17/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES					
PLATE	GRIP(DRY)	SHEAR	SECTION		
	(PSI)	(PLI)	(PLI)		
	MAX	MIN	MAX	MIN	MAX
MT20	650	371	1747	788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

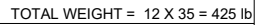
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (B) (INPUT = 0.90)
JSI METAL= 0.21 (B) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

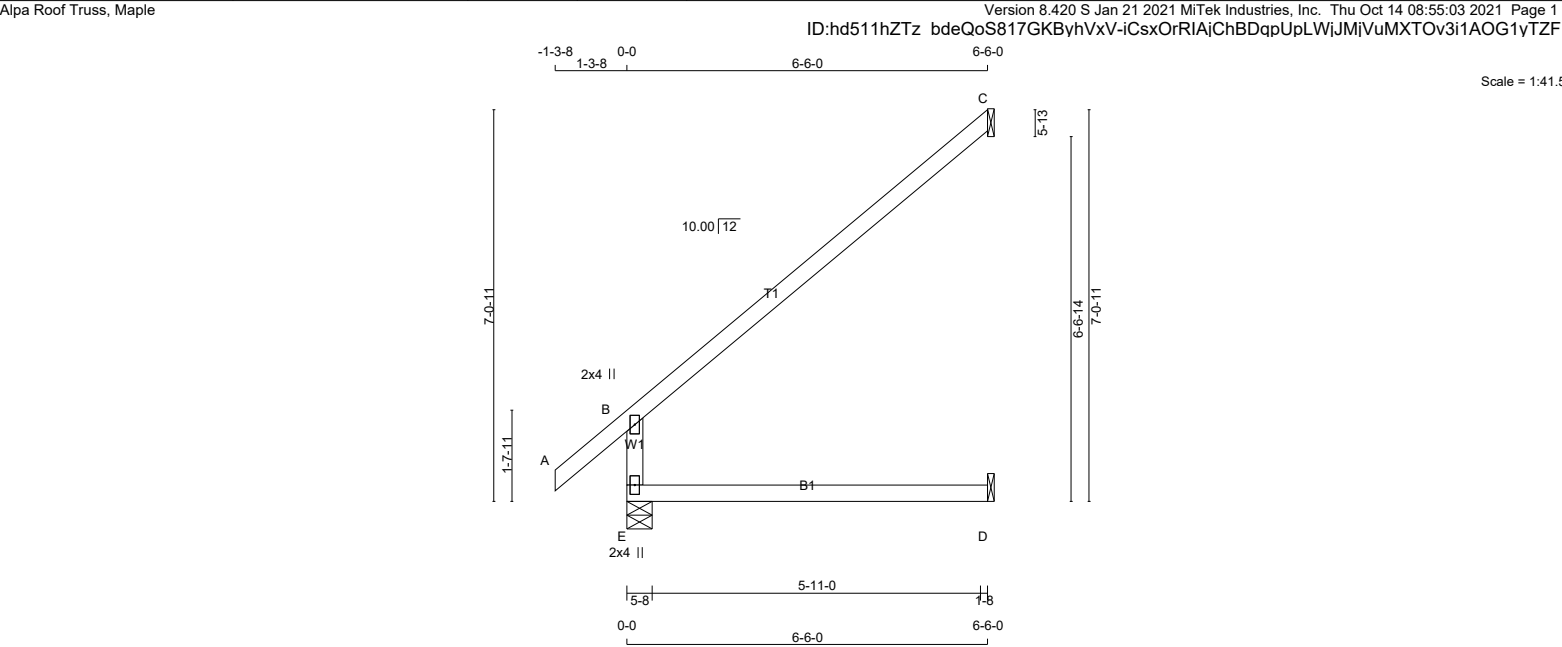


Scale = 1:38.6



JSI GRIP= 0.33 (B) (INPUT = 0.90)
JSI METAL= 0.08 (I) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
336335	J02		1	TRUSS DESC. JT 45147	E21104233



TOTAL WEIGHT = 36 X 21 = 743 lb

[M][F]

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	2.0	4.0	
E	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
E	494	0	494	0	5-8	1-8
C	190	0	190	0	1-8	1-8
D	51	0	57	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS				DEAD	SOIL
	SNOW	LIVE	PERM.LIVE	WIND				
E	350	229 / 0	0 / 0	0 / 0	0 / 0	121 / 0	0 / 0	
C	132	102 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0	
D	41	0 / 0	0 / 0	0 / 0	0 / 0	41 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)		FACTORED VERT. LOAD (PLF)		MAX LC1	MAX LC2	MAX UNBRAC LENGTH	MEMB. FR-TO	WEBS MAX. FACTORED FORCE (LBS)	
	FROM	TO	FROM	TO					FROM	TO
E-B	-425 / 0		0.0	0.0	0.13 (4)		7.81			
A-B	0 / 35		-78.0	-78.0	0.11 (1)		10.00			
B-C	-41 / 0		-78.0	-78.0	0.44 (1)		6.25			
E-D	0 / 0		-18.5	-18.5	0.16 (4)		10.00			

DESIGN CRITERIA

SPECIFIED LOADS:			
TOP CH.	LL =	21.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	34.4	PSF

SPACING = 24.0 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.44/1.00 (B-C:1) , BC=0.16/1.00 (D-E:4) ,
WB=0.00/1.00 (n/a:0) , SSI=0.19/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES					
PLATE	GRIP(DRY)	SHEAR	SECTION		
	(PSI)	(PLI)	(PLI)		
	MAX	MIN	MAX	MIN	
MT20	650	371	1747	788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

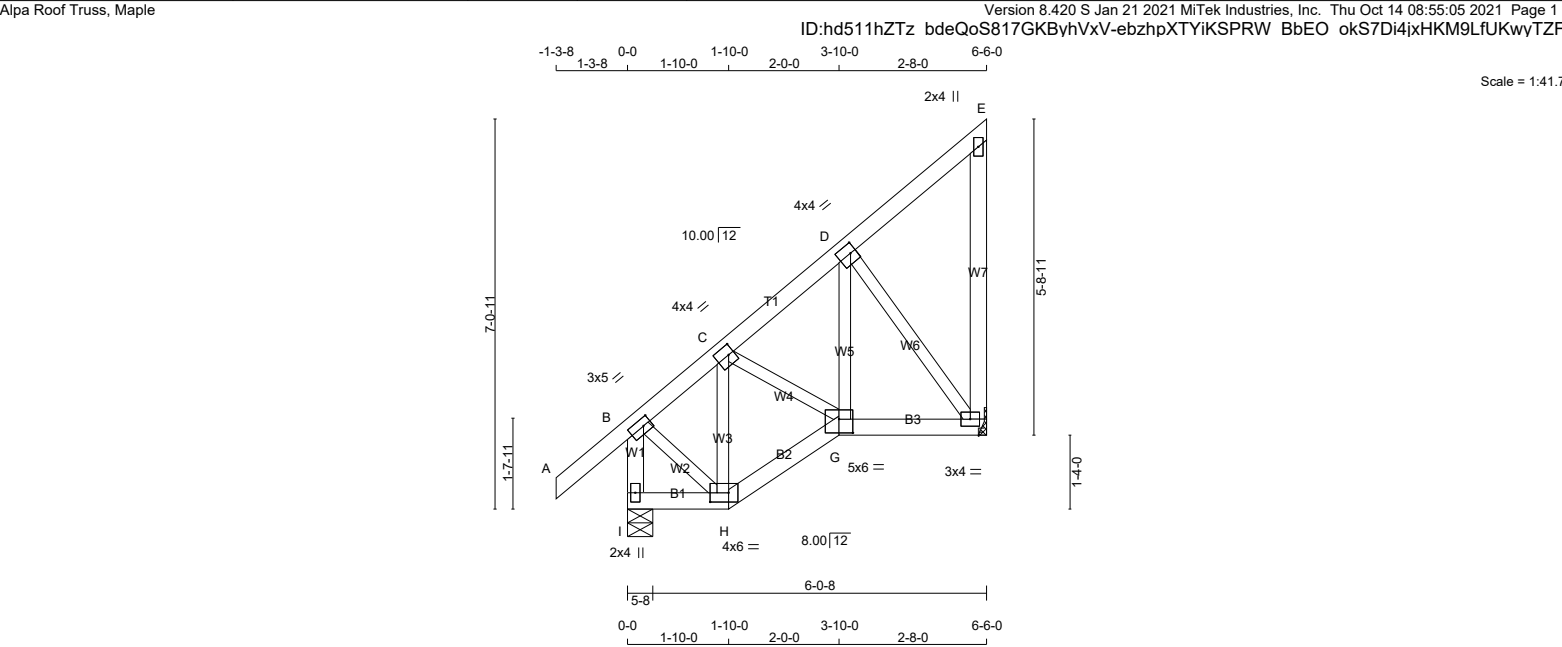
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.29 (B) (INPUT = 0.90)
JSI METAL= 0.23 (B) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
336335	J02T		1	TRUSS DESC. JT 45147	E21104234



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
I - B	2x4	DRY No.2	SPF
A - E	2x4	DRY No.2	SPF
F - E	2x4	DRY No.2	SPF
I - H	2x4	DRY No.2	SPF
H - G	2x4	DRY No.2	SPF
G - F	2x4	DRY No.2	SPF
ALL WEBS 2x3 DRY No.2 SPF			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-t	MT20	3.0	5.0	1.50 1.75
C	TMWW-t	MT20	4.0	4.0	2.00 1.25
D	TMWW-t	MT20	4.0	4.0	2.00 1.25
E	TMV+p	MT20	2.0	4.0	
F	BMVW1-t	MT20	3.0	4.0	
G	BBWW-I	MT20	5.0	6.0	3.00 3.00
H	BBWW-I	MT20	4.0	6.0	2.00 4.00
I	BMV1+p	MT20	2.0	4.0	

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

Y. WIDYA
100225448
10/14/2021
PROVINCE OF ONTARIO

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
JT	433	0	433	0	5-8	1-8
F	302	0	302	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS					WIND	DEAD	SOIL
		SNOW	LIVE	PERM.LIVE					
I	306	201 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	105 / 0	0 / 0
F	216	130 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	85 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)		
FR-TO		FROM TO			FR-TO				
I-B	-416 / 0	0.0	0.0	0.04 (1)	7.81	B-H	0 / 159	0.04 (1)	
A-B	0 / 35	-78.0	-78.0	0.11 (1)	10.00	H-C	-139 / 0	0.03 (1)	
B-C	-195 / 0	-78.0	-78.0	0.08 (1)	6.25	C-G	0 / 43	0.01 (1)	
C-D	-194 / 0	-78.0	-78.0	0.06 (1)	6.25	G-D	0 / 104	0.03 (4)	
D-E	-13 / 0	-78.0	-78.0	0.06 (1)	6.25	D-F	-253 / 0	0.07 (1)	
F-E	-79 / 0	0.0	0.0	0.05 (1)	7.81				
I-H	0 / 0	-18.5	-18.5	0.02 (4)	10.00				
H-G	0 / 150	-18.5	-18.5	0.04 (4)	10.00				
G-F	0 / 157	-18.5	-18.5	0.05 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	34.4	PSF	

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.11/1.00 (A-B:1) , BC=0.05/1.00 (F-G:4) , WB=0.07/1.00 (D-F:1) , SSI=0.07/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

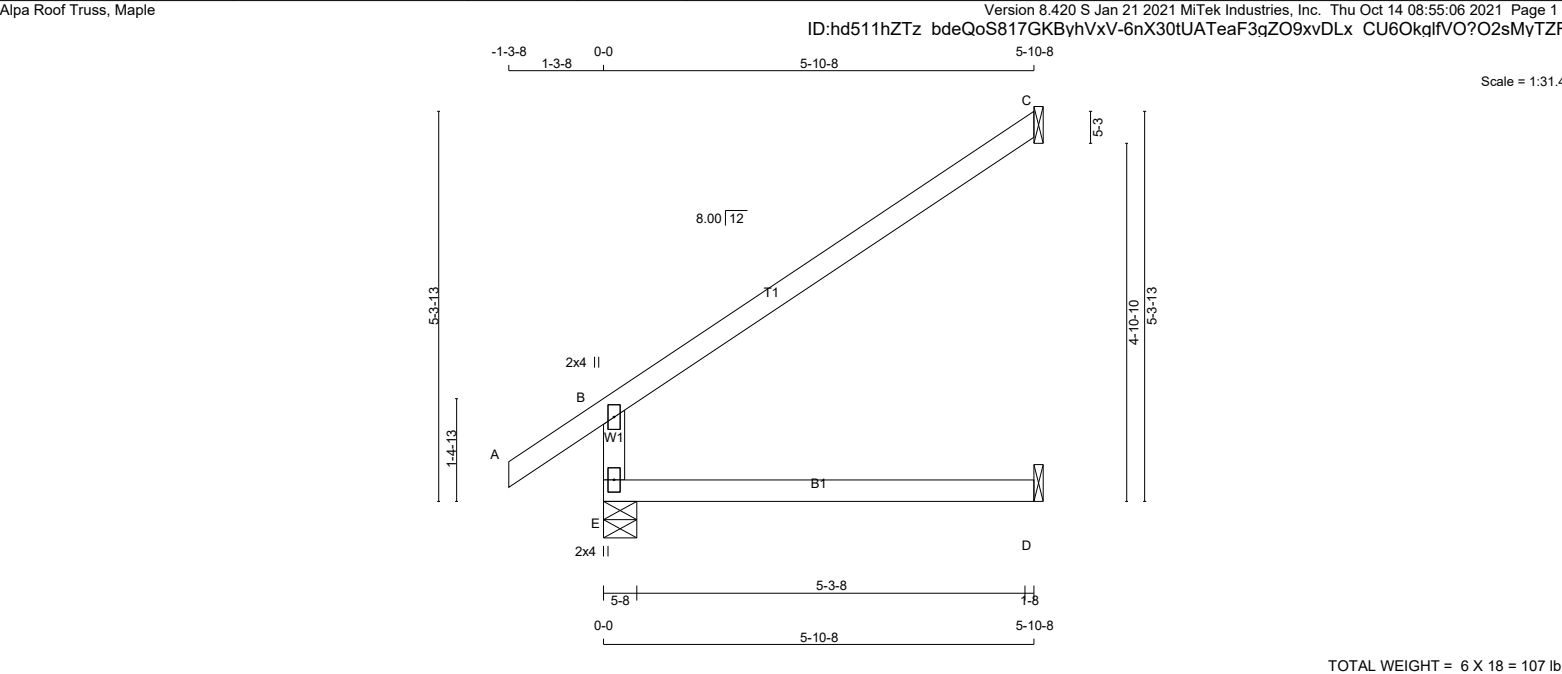
NAIL VALUES					
PLATE GRIP(DRY)	SHEAR	SECTION			
(PSI)	(PLI)	(PLI)			
MAX MIN	MAX MIN	MAX MIN			
MT20	650	371	1747	788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.35 (B) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
336335	J04		1	TRUSS DESC. JT 45147	E21104235



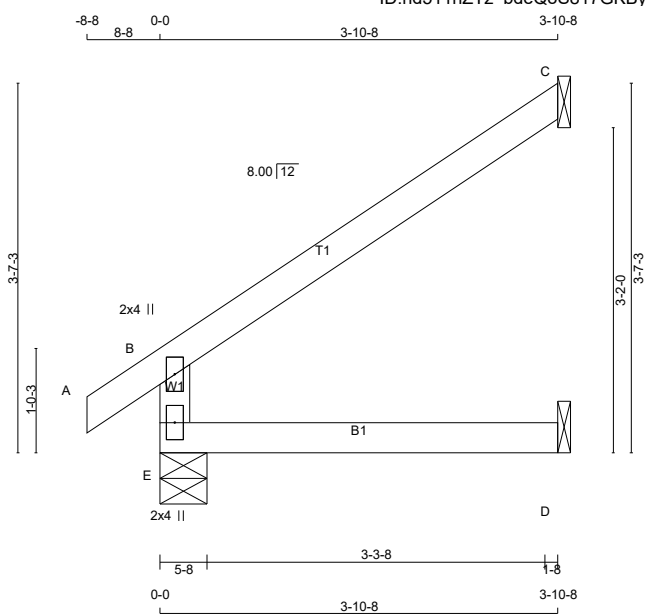
LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. E - B 2x4 DRY No.2 SPF A - C 2x4 DRY No.2 SPF E - D 2x4 DRY No.2 SPF DRY: SEASONED LUMBER.							DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS <table><tr><th></th><th colspan="2">FACTORED GROSS REACTION</th><th colspan="2">MAXIMUM FACTORED GROSS REACTION</th><th colspan="2">INPUT BRG</th><th colspan="2">REQRD BRG</th></tr><tr><th>JT</th><th>VERT</th><th>HORZ</th><th>DOWN</th><th>HORZ</th><th>UPLIFT</th><th>IN-SX</th><th>IN-SX</th><th></th></tr><tr><td>E</td><td>456</td><td>0</td><td>456</td><td>0</td><td>0</td><td>5-8</td><td>1-8</td><td></td></tr><tr><td>C</td><td>172</td><td>0</td><td>172</td><td>0</td><td>0</td><td>1-8</td><td>1-8</td><td></td></tr><tr><td>D</td><td>46</td><td>0</td><td>51</td><td>0</td><td>0</td><td>1-8</td><td>1-8</td><td></td></tr></table> SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D UNFACTORED REACTIONS <table><tr><th>JT</th><th>1ST LCASE</th><th colspan="6">MAX./MIN. COMPONENT REACTIONS</th></tr><tr><th>JT</th><th>COMBINED</th><th>SNOW</th><th>LIVE</th><th>PERM.LIVE</th><th>WIND</th><th>DEAD</th><th>SOIL</th></tr><tr><td>E</td><td>323</td><td>212 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>111 / 0</td><td>0 / 0</td></tr><tr><td>C</td><td>119</td><td>93 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>26 / 0</td><td>0 / 0</td></tr><tr><td>D</td><td>36</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>36 / 0</td><td>0 / 0</td></tr></table> BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (4) <table><tr><th colspan="4">CHORDS</th><th colspan="4">WEBS</th></tr><tr><th>MEMB.</th><th>MAX. FACTORED FORCE (LBS)</th><th>FACTORED VERT. LOAD (PLF)</th><th>LC1 MAX CSI (LC)</th><th>MAX. UNBRAC LENGTH</th><th>MEMB.</th><th>MAX. FACTORED FORCE (LBS)</th><th>MAX CSI (LC)</th></tr><tr><td>FR-TO</td><td></td><td>FROM</td><td>TO</td><td></td><td>FR-TO</td><td></td><td></td></tr><tr><td>E- B</td><td>-393 / 0</td><td>0.0</td><td>0.0</td><td>0.12 (4)</td><td>7.81</td><td></td><td></td></tr><tr><td>A- B</td><td>0 / 30</td><td>-78.0</td><td>-78.0</td><td>0.10 (1)</td><td>10.00</td><td></td><td></td></tr><tr><td>B- C</td><td>-32 / 0</td><td>-78.0</td><td>-78.0</td><td>0.46 (1)</td><td>6.25</td><td></td><td></td></tr><tr><td>E- D</td><td>0 / 0</td><td>-18.5</td><td>-18.5</td><td>0.13 (4)</td><td>10.00</td><td></td><td></td></tr></table>											FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG		JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX		E	456	0	456	0	0	5-8	1-8		C	172	0	172	0	0	1-8	1-8		D	46	0	51	0	0	1-8	1-8		JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS						JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	E	323	212 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0	C	119	93 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0	D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0	CHORDS				WEBS				MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	FR-TO		FROM	TO		FR-TO			E- B	-393 / 0	0.0	0.0	0.12 (4)	7.81			A- B	0 / 30	-78.0	-78.0	0.10 (1)	10.00			B- C	-32 / 0	-78.0	-78.0	0.46 (1)	6.25			E- D	0 / 0	-18.5	-18.5	0.13 (4)	10.00			DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 21.0 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.4 PSF TOTAL LOAD = 34.4 PSF SPACING = 24.0 IN.C/C THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018 , ABC 2019 - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014 DESIGN ASSUMPTIONS -OVERHANG NOT TO BE ALTERED OR CUT OFF. (55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL)= L/360 (0.20") CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00") ALLOWABLE DEFL.(TL)= L/360 (0.20") CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03") CSI: TC=0.46/1.00 (B-C:1) , BC=0.13/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.19/1.00 (B-C:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10 COMPANION LIVE LOAD FACTOR = 1.00 AUTOSOLVE RIGHT HEEL ONLY TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT . NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.24 (B) (INPUT = 0.90) JSI METAL= 0.20 (B) (INPUT = 1.00)						
	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG																																																																																																																																																													
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX																																																																																																																																																													
E	456	0	456	0	0	5-8	1-8																																																																																																																																																													
C	172	0	172	0	0	1-8	1-8																																																																																																																																																													
D	46	0	51	0	0	1-8	1-8																																																																																																																																																													
JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS																																																																																																																																																																		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL																																																																																																																																																													
E	323	212 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0																																																																																																																																																													
C	119	93 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0																																																																																																																																																													
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0																																																																																																																																																													
CHORDS				WEBS																																																																																																																																																																
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)																																																																																																																																																													
FR-TO		FROM	TO		FR-TO																																																																																																																																																															
E- B	-393 / 0	0.0	0.0	0.12 (4)	7.81																																																																																																																																																															
A- B	0 / 30	-78.0	-78.0	0.10 (1)	10.00																																																																																																																																																															
B- C	-32 / 0	-78.0	-78.0	0.46 (1)	6.25																																																																																																																																																															
E- D	0 / 0	-18.5	-18.5	0.13 (4)	10.00																																																																																																																																																															
PLATES (table is in inches) <table><tr><th>JT</th><th>TYPE</th><th>PLATES</th><th>W</th><th>LEN</th><th>Y</th><th>X</th></tr><tr><td>B</td><td>TMV+p</td><td>MT20</td><td>2.0</td><td>4.0</td><td></td><td></td></tr><tr><td>E</td><td>BMV1+p</td><td>MT20</td><td>2.0</td><td>4.0</td><td></td><td></td></tr></table>							JT	TYPE	PLATES	W	LEN	Y	X	B	TMV+p	MT20	2.0	4.0			E	BMV1+p	MT20	2.0	4.0			LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2 																																																																																																																																								
JT	TYPE	PLATES	W	LEN	Y	X																																																																																																																																																														
B	TMV+p	MT20	2.0	4.0																																																																																																																																																																
E	BMV1+p	MT20	2.0	4.0																																																																																																																																																																

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
336335	J08		1	TRUSS DESC. JT 45147	E21104236

Alpa Roof Truss, Maple

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Oct 14 08:55:08 2021 Page 1

ID:hd511hZTz bdeQoS817GKByhVxV-3AfgRZVR?FqzI jmGMxhQM4dlv5M8f9ortt9xFyTZFH



TOTAL WEIGHT = 2 X 12 = 23 lb

[M][F]

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	2.0	4.0	
E	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
JT	292	0	292	0	5-8	1-8
E	113	0	113	0	1-8	1-8
C	30	0	34	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	207	135 / 0	0 / 0	0 / 0	0 / 0	72 / 0	0 / 0
C	78	61 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	24	0 / 0	0 / 0	0 / 0	0 / 0	24 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO		FR-TO		
E-B	-250 / 0	0.0	0.0	0.04 (4)	7.81		
A-B	0 / 17	-78.0	-78.0	0.04 (1)	10.00		
B-C	-21 / 0	-78.0	-78.0	0.15 (1)	6.25		
E-D	0 / 0	-18.5	-18.5	0.06 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.15/1.00 (B-C:1) , BC=0.06/1.00 (D-E:4) ,
WB=0.00/1.00 (n/a:0) , SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (B) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)

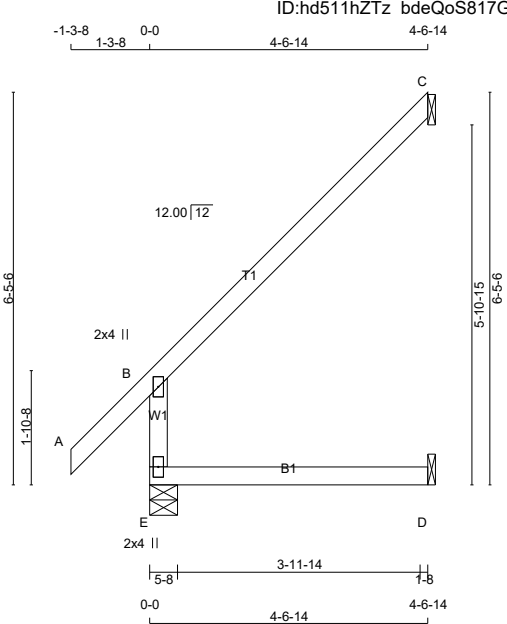
LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
336335	J09		1	TRUSS DESC. JT 45147	E21104237

Alpa Roof Truss, Maple

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Oct 14 08:55:09 2021 Page 1



LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	2.0	4.0	
E	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	379	0	379	0	5-8	1-8
C	134	0	134	0	1-8	1-8
D	37	0	41	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS				DEAD	SOIL
	SNOW	LIVE	PERM.LIVE	WIND				
E	268	178 / 0	0 / 0	0 / 0	0 / 0	89 / 0	0 / 0	0 / 0
C	93	72 / 0	0 / 0	0 / 0	0 / 0	21 / 0	0 / 0	0 / 0
D	30	0 / 0	0 / 0	0 / 0	0 / 0	30 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)		FACTORED VERT. LOAD (PLF)		MAX LC1 (LC)	MAX UNBRAC LENGTH	MEMB. FR-TO	WEBS MAX. FACTORED FORCE (LBS)		MAX CSI (LC)
	FROM	TO	FROM	TO				FROM	TO	
E-B	-331 / 0		0.0	0.0	0.06 (4)	7.81				
A-B	0 / 38		-78.0	-78.0	0.11 (1)	10.00				
B-C	-31 / 0		-78.0	-78.0	0.28 (1)	6.25				
E-D	0 / 0		-18.5	-18.5	0.09 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:		
TOP CH.	LL = 21.0	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 34.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.28/1.00 (B-C:1) , BC=0.09/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES					
PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)			
	MAX MIN	MAX MIN	MAX MIN		
MT20	650 371	1747 788	1987 1873		

PLATE PLACEMENT TOL. = 0.250 inches

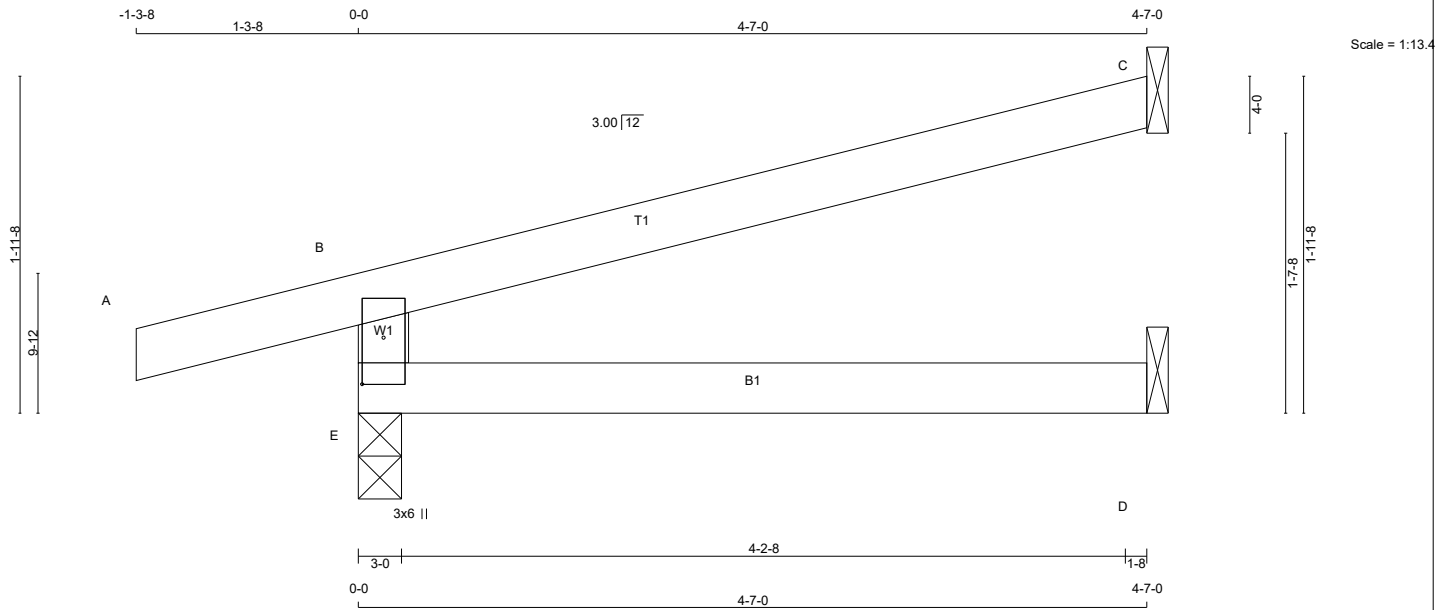
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (B) (INPUT = 0.90)
JSI METAL= 0.17 (B) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

Alpa Roof Truss, Maple

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Oct 14 08:55:11 2021 Page 1
ID:hd511hZTz bdeQoS817GKByhVxV-TILy3aYJHACY9RSLyUVO2?h6176dL0uEXG6pYzYZFE



TOTAL WEIGHT = 9 X 13 = 113 lb

N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY No.2	SPF
A - C	2x4	DRY No.2	SPF
E - D	2x4	DRY No.2	SPF

DRY: SEASONED LUMBER.

JT	TYPE	PLATES	W	LEN	Y	X
B						
E	TMBMV1+p	MT20	3.0	6.0	3.25	1.50

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	377	0	377	0	0	3-0	1-8
C	134	0	134	0	0	1-8	1-8
D	34	0	39	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	266	176 / 0	0 / 0	0 / 0	0 / 0	91 / 0	0 / 0
C	93	72 / 0	0 / 0	0 / 0	0 / 0	21 / 0	0 / 0
D	28	0 / 0	0 / 0	0 / 0	0 / 0	28 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED UNBRAC. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED UNBRAC. CSI (LC)	
FR-TO		FROM TO		FR-TO			
E-B	-327 / 0	0.0 0.0	0.09 (4)	7.81			
A-B	0 / 13	-78.0 -78.0	0.10 (1)	10.00			
B-C	-11 / 0	-78.0 -78.0	0.28 (1)	6.25			
E-D	0 / 0	-18.5 -18.5	0.09 (4)	10.00			

LATERAL BRACE(S) SHOWN SHALL BE
2X4 SPF#2



SPECIFIED LOADS:

TOP	CH.	LL	=	21.0	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.4	PSF
TOTAL LOAD				=	34.4 PSF

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.28/1.00 (B-C:1) , BC=0.09/1.00 (D-E:4) ,
WB=0.00/1.00 (n/a:0) , SSI=0.17/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES						
PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (E) (INPUT = 0.90)
JSI METAL= 0.05 (E) (INPUT = 1.00)

LUS – Double Shear Joist Hangers



All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

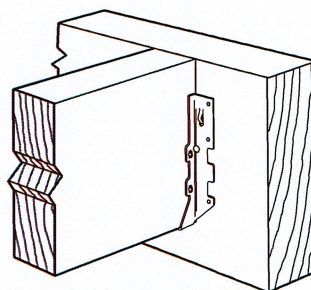
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

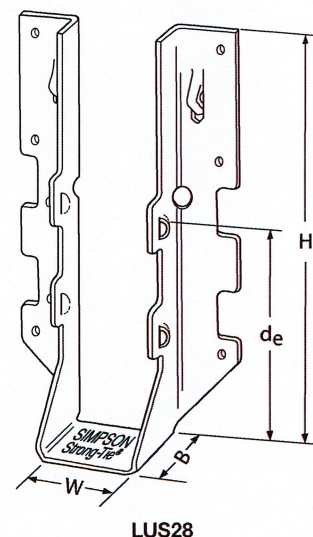
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified



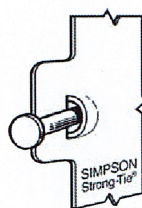
Typical LUS Installation



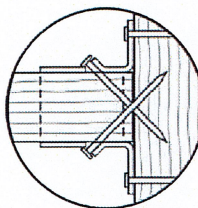
LUS28

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _p =1.15)	Normal (K _p =1.00)	Uplift (K _p =1.15)	Normal (K _p =1.00)
LUS24	18	1½	3½	1¾	1 15/16	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3½	3½	2	1 13/16	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1½	4¾	1¾	3½	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3½	4¾	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4½	4¾	2	3¼	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1½	6½	1¾	3¾	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3½	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4½	6¼	2	3¼	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1½	7 15/16	1¾	3¾	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3½	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4½	8¾	2	5¼	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,580



Double Shear Nailing Top View.



LIMIT
STATES
DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

© 2020 Simpson Strong-Tie Company Inc.

T-SPECLUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

HUS/LJS – Double Shear Joist Hangers



All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

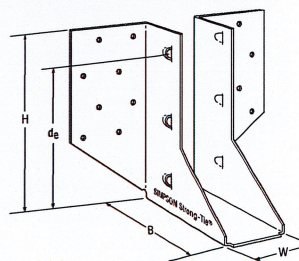
- Factored resistances are in accordance with CSA O86 -14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

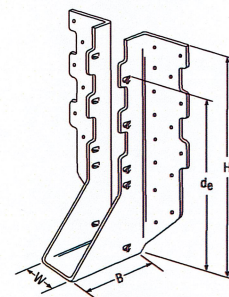
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

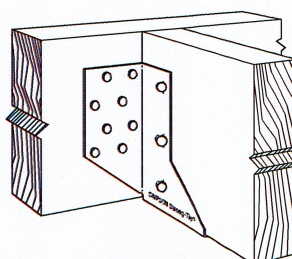
- See current catalogue for options



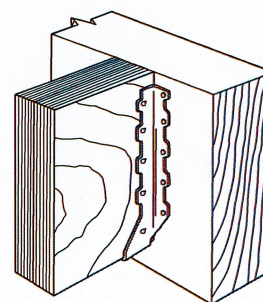
LJS26DS



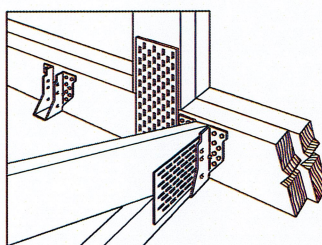
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



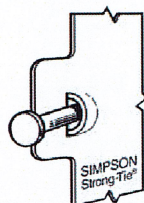
Typical HUS
Installation



Typical HUS Installation
(Truss Designer to provide fastener
quantity for connecting multiple
members together)

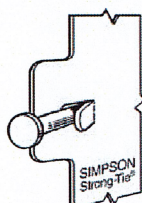
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K ₀ =1.15) lb.	Normal (K ₀ =1.00) lb.	Uplift (K ₀ =1.15) lb.	Normal (K ₀ =1.00) lb.
LJS26DS	18	1½	5	3½	4⅞	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1⅝	5⅞	3	3½	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1⅝	7⅜	3	6⅜	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1⅝	9⅜	3	7⅜	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1⅜	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

1. d_e is the distance from the seat of the hanger to the highest joist nail.

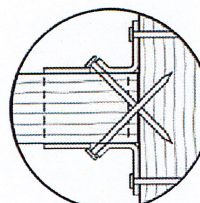


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent
5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT
STATES
DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

© 2020 Simpson Strong-Tie Company Inc.

T-SPECHUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

HHUS – Double Shear Joist Hangers

All HHUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 14 gauge

Finish: G90 galvanized

Design:

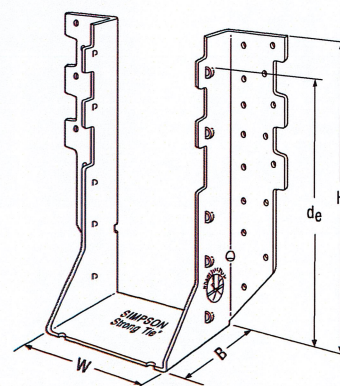
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

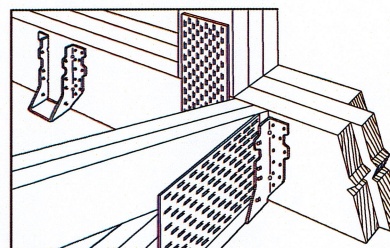
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

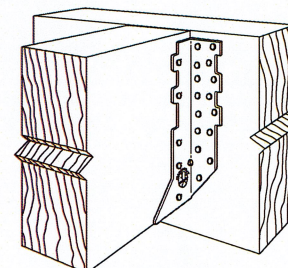
- See current catalogue for options



HHUS410



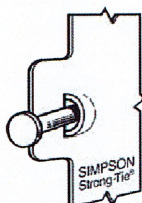
Typical HHUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)



Typical HHUS Installation

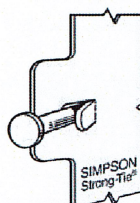
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _o =1.15)	Normal (K _o =1.00)	Uplift (K _o =1.15)	Normal (K _o =1.00)
HHUS26-2	14	3 5/16	5 13/16	3	3 15/16	(14) 16d	(6) 16d	2850	7335	2065	5205
HHUS28-2	14	3 5/16	7 1/2	3	6 5/32	(22) 16d	(8) 16d	3765	8940	2675	6345
HHUS210-2	14	3 5/16	9 3/32	3	8	(30) 16d	(10) 16d	4670	9660	4235	7000
HHUS210-3	14	4 11/16	9	3	7 15/16	(30) 16d	(10) 16d	4670	9670	4235	6865
HHUS210-4	14	6 1/8	8 29/32	3	7 27/32	(30) 16d	(10) 16d	4670	10155	4235	7210
HHUS46	14	3 3/8	5 13/32	3	3 15/16	(14) 16d	(6) 16d	2540	7335	2065	5205
HHUS48	14	3 3/8	7 1/8	3	6 1/8	(22) 16d	(8) 16d	3765	8940	2675	6345
HHUS410	14	3 3/8	9	3	8	(30) 16d	(10) 16d	4670	9855	4235	7000
HHUS5.50/10	14	5 1/2	9	3	8	(30) 16d	(10) 16d	4670	10155	4235	7210
HHUS7.25/10	14	7 1/4	9	3 3/8	7 29/32	(30) 16d	(10) 16d	4670	10155	3370	7210

1. d_e is the distance from the seat of the hanger to the highest joist nail.

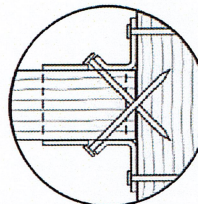


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent
5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



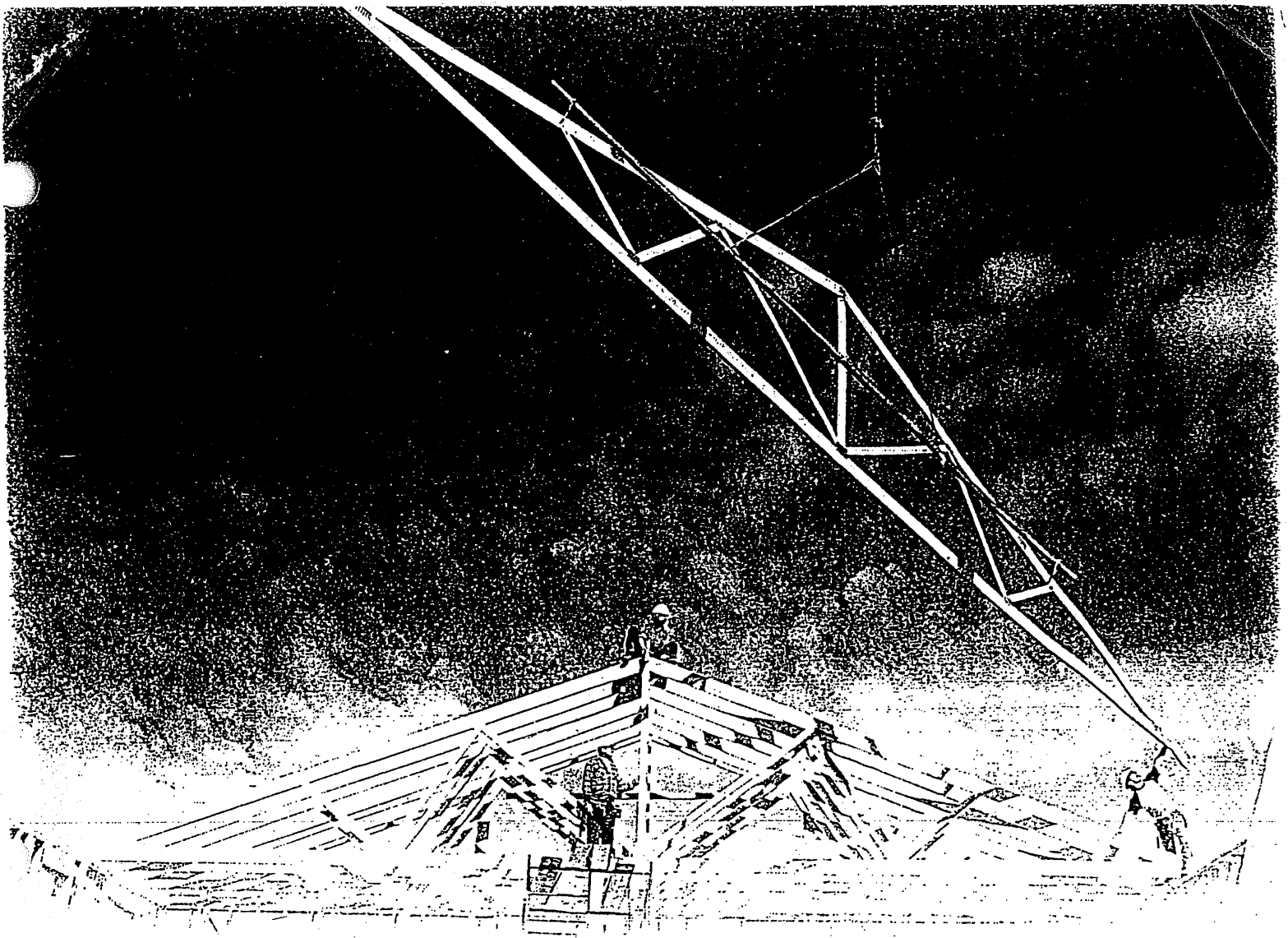
LIMIT
STATES
DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

© 2020 Simpson Strong-Tie Company Inc.

T-SPECHHUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com



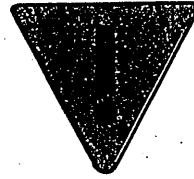
Wood Truss Installation

**A Guide to proper handling, erecting and bracing
metal plate connected wood trusses**

Table of Content

Warning.....	4
1 Unloading & Lifting.....	5
2 Job Site Handling.....	5
3 Hoisting.....	6
4 Beginning the Erection Process.....	7
5 Erection Tolerance.....	8
6 Bracing.....	8
7 Bracing Requirements for 3 Planes of Roof.....	9
8 Stacking Materials.....	10
Caution Notes.....	11

Warning



General

Familiarity with the Construction Design Documents, the Truss Design Drawings, and Truss Placement Plans (if required by the Construction Design Documents) is required to properly erect, brace, and connect the trusses to the building system.

All of the care and quality involved in the design and manufacture of wood trusses can be jeopardized if the trusses are not properly handled, erected, and braced.

The consequences of improper handling, erecting, and bracing may be a collapse of the structure, which at best is a substantial loss of time and materials, and at worst is a loss of life. The majority of truss accidents occur during truss installation and not as a result of improper design or manufacture.

Prior to truss erection, the builder/erector shall meet with the erection crew for a safety and planning meeting, making sure each crew member understands his or her roles and responsibilities during the erection process.

Temporary Erection Bracing

Trusses are not marked in any way to identify the frequency, or location of temporary erection bracing.

All temporary bracing shall comply with the latest edition of *Commentary and Recommendations for Handling, Installing & Bracing Metal Plate Connected Wood Trusses* (HIB), published by the Truss Plate Institute, and/or as specified in the Construction Design Documents prepared by the building designer.

Permanent Truss Bracing

Permanent bracing for the roof or floor trusses is the responsibility of the building designer and should be shown on the Construction Design Documents. Permanent bracing locations for individual compression members of a wood truss are shown on the Truss Design Drawings, and shall be installed by the building or erection contractor. This bracing is needed for the proper performance of individual trusses within the roof or floor system. The design and connection of the bracing to the truss and then to the overall building system is the responsibility of the building designer, and is in addition to the permanent bracing plan, which is also specified by the building designer.

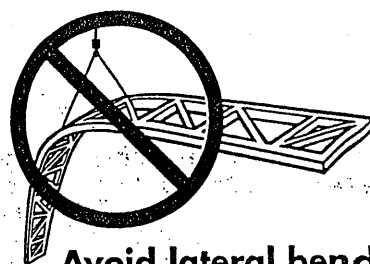
Special Design Requirements

Special design requirements, such as wind bracing, portal bracing, seismic bracing, diaphragms, shear walls, or other load transfer elements and their connections to wood trusses must be considered separately by the building designer, who shall determine size, location, and method of connections for all bracing as needed to resist these forces.

1 Unloading & Lifting

Never handle trusses flat

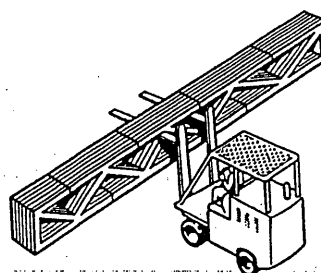
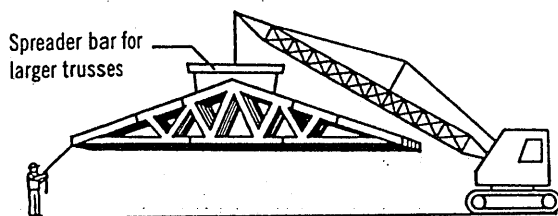
Beginning with the unloading process, and throughout all phases of construction, care must be taken to avoid lateral bending of trusses, which can cause damage to the lumber and metal connector plates at the joints.



Avoid lateral bending

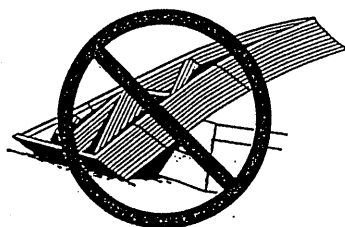
- Use special care in windy weather.
- If using a crane within 10 feet of an electric line, contact the local power company.
- If using a crane within 5 miles of an airport, contact the airport 30 days prior to erection to learn about any safety regulations that must be followed.

2 Job Site Handling



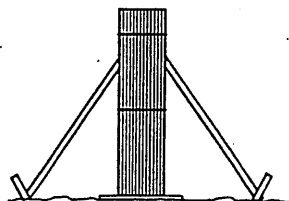
All trusses should be picked up at the top chords in a vertical position only

Proper banding and smooth ground allow for unloading of trusses without damage. This should be done as close to the building site as possible to minimize handling. Do not break banding until installation begins. Hand erection of trusses is allowed, provided excessive lateral bending is prevented.



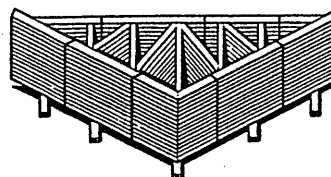
Do not store unbraced bundles upright

If trusses are stored vertically they shall be braced in a manner that will prevent tipping or topping. Generally cutting of the banding is done just prior to installation.



Do not store on uneven ground

If trusses are stored horizontally, blocking should be used on eight to ten foot centers, or as required, to minimize lateral bending and moisture gain.

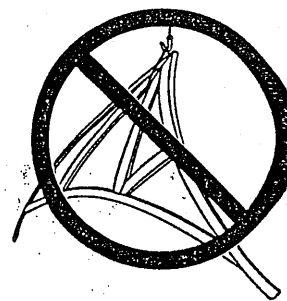


Care should be exercised when removing banding to avoid damaging trusses.

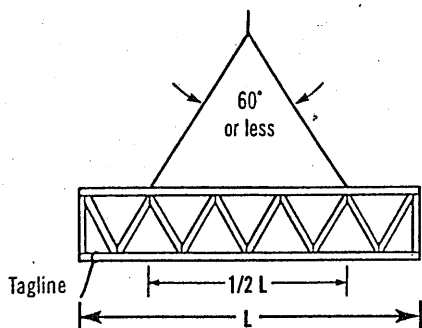
During long term storage, trusses shall be protected from the environment in a manner that provides for adequate ventilation of the trusses. If tarpaulins or other material is used, the ends shall be left open for ventilation. Plastic is not recommended, since it can trap moisture.

3 Hoisting

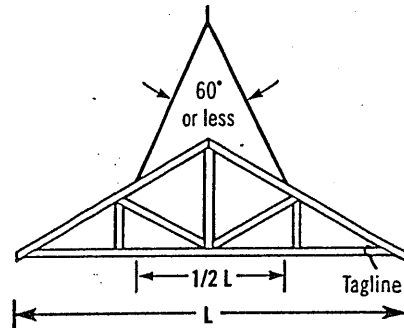
All trusses that are erected one at a time shall be held safely in position by the erection equipment until such time as all necessary bracing has been installed and the ends of the trusses are securely fastened to the building.



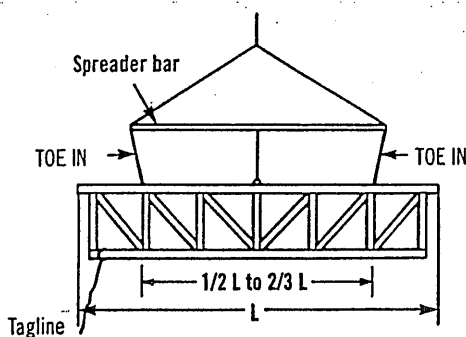
Avoid lateral bending



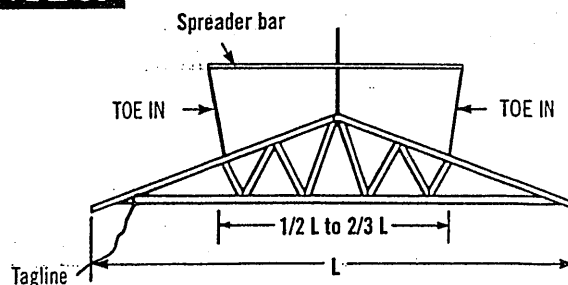
$L \leq 30'$



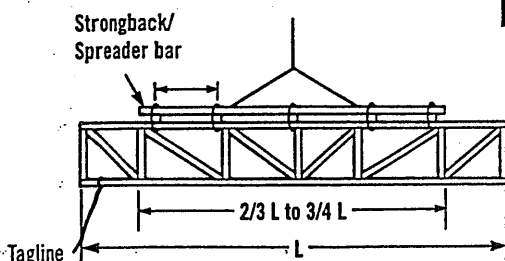
Truss sling is acceptable where these criteria are met.



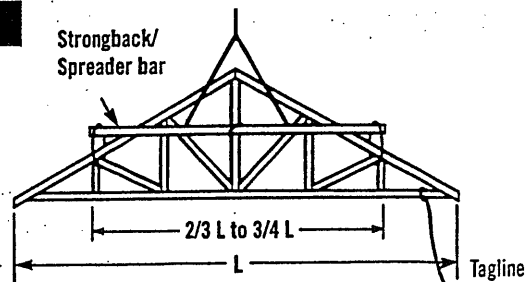
$30' < L \leq 60'$



Use spreader bar in all other cases. It should be noted that the lines from the ends of the spreader bar "TOE IN"; if these lines should "TOE OUT" the truss may fold in half.



$L > 60'$



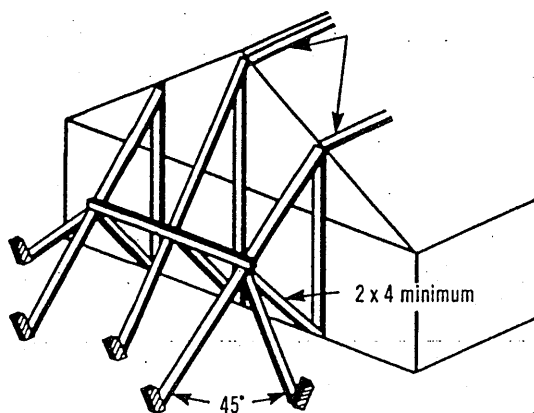
For lifting trusses with spans in excess of 60 feet, it is recommended that a strongback/spreader bar be used as illustrated. The strongback/spreader bar should be attached to the top chord and web members at intervals of approximately 10 feet. Further, the strongback/spreader bar should be at or above the mid-height of the truss to prevent overturning. The strongback/spreader bar can be of any material with sufficient strength to safely carry the weight of the truss and sufficient rigidity to adequately resist bending of the truss.

4 Beginning the Erection Process

It is important for the builder or erection contractor to provide substantial bracing for the first truss erected. The two or more trusses making up the rest of the first set are tied to and rely upon the first truss for stability. Likewise, after this first set of trusses is adequately cross-braced, the remaining trusses installed rely upon this first set for stability. Thus, the performance of the truss bracing system depends to a great extent on how well the first group of trusses is braced.

Ground Brace - Exterior

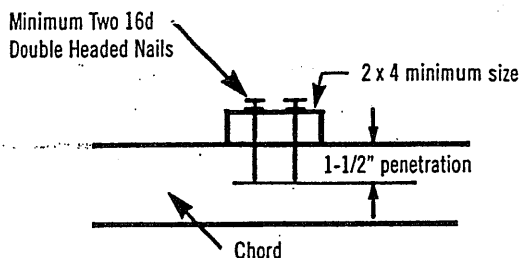
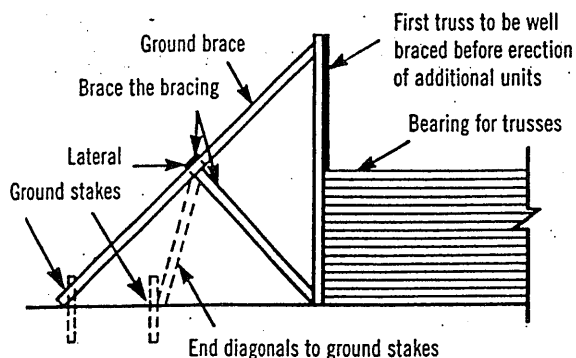
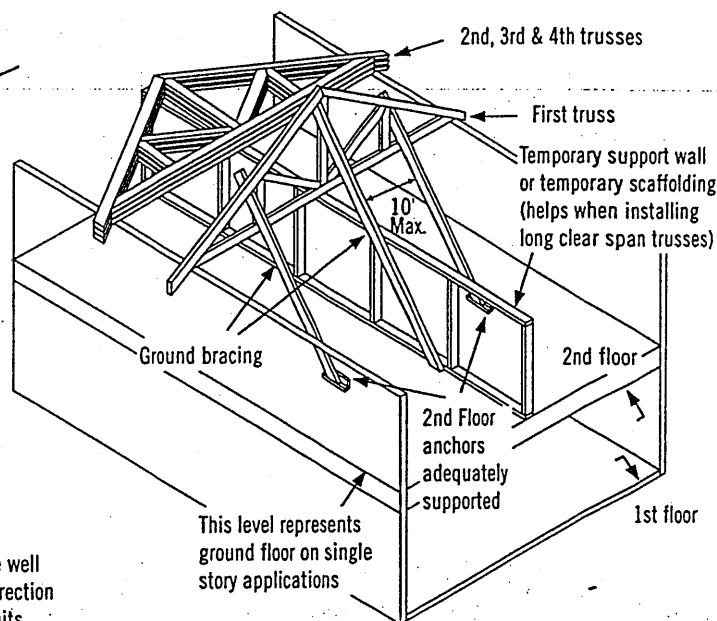
One satisfactory method ties the first unit of trusses off to a series of braces that are attached to a stake driven into the ground and securely anchored. The ground brace itself should be supported as shown below or it is apt to buckle. Additional ground braces in the opposite direction, inside the building, are also recommended.



Note: Locate ground braces for first truss directly in line with all rows of top chord continuous lateral bracing (either temporary or permanent).

Ground Brace - Interior

Another satisfactory method where height of building or ground conditions prohibit bracing from the exterior is to tie the first truss rigidly in place from the interior at the floor level, provided the floor is substantially completed and capable of supporting the ground bracing forces. Securely fasten the first truss to the middle of the building. Brace the bracing similar to exterior ground bracing shown at left. Set trusses from the middle toward the end of the building. Properly cross-brace the first set of trusses before removing floor braces and setting remaining trusses.

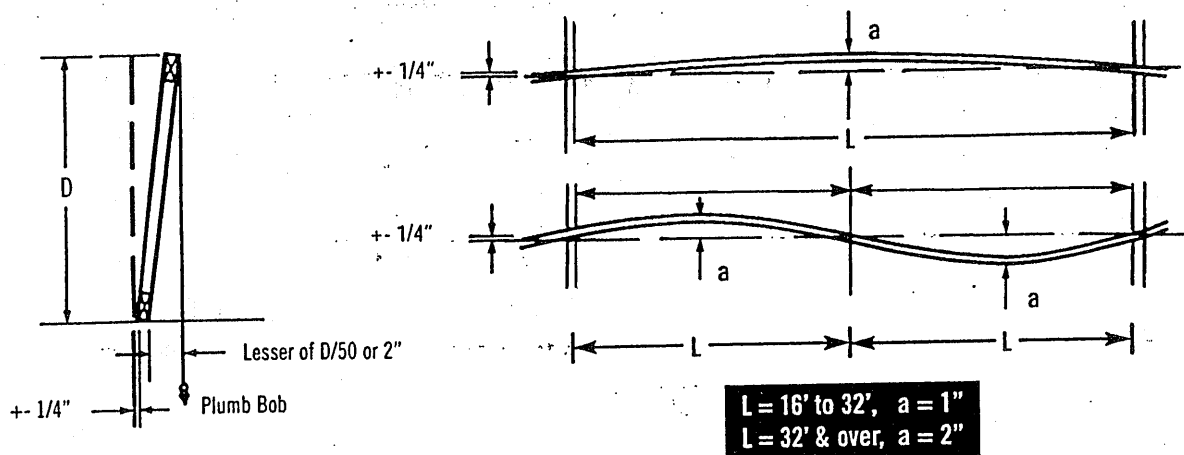


Inadequate size of bracing material or inadequate fastening is a major cause of erection dominoing.

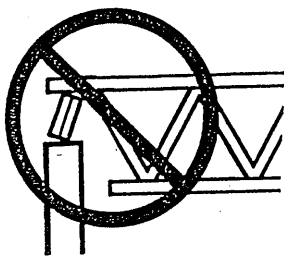
5 Erection Tolerance

Complying with erection tolerances is critical to achieving an acceptable roof or floor line, and to accomplishing effective bracing. Setting trusses within tolerance the first time will prevent the need for the hazardous practice of respacing or adjusting trusses when roof sheathing or roof purlins are installed. Trusses leaning or bowing can cause nails to miss the top chords when sheathing is applied, and create cumulative stresses on the bracing, which is a frequent cause of dominoing.

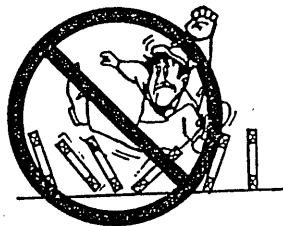
When sheathing, make sure nails are driven into the top chord of the trusses.



6 Bracing



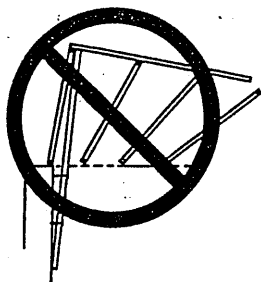
Do not install trusses on temporarily connected supports



Do not walk on unbraced trusses



Do not walk on trusses or gable ends lying flat

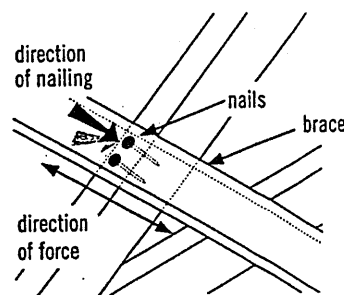


Nails in withdrawal (parallel to force)

All anchors, hangers, tie-downs, seats, bearing ledgers, etc., that are part of the supporting structure shall be accurately and properly placed and permanently attached before truss installation begins. No trusses shall ever be installed on anchors or ties that have temporary connections to the supporting structure.

Nailing scabs to the end of the building to brace the first truss is not recommended.

All nailing of bracing should be done so that nails are driven perpendicular to the direction of force, as shown at right.



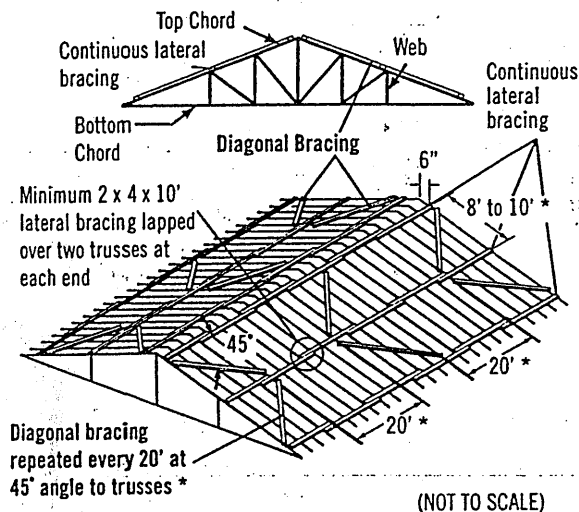
Well nailed (perpendicular to force)

7 Bracing Requirements for 3 Planes of Roof

Temporary erection bracing must be applied to three planes of the roof system to ensure stability: Plane 1) Top Chord (sheathing), Plane 2) Bottom Chord (ceiling plane), and Plane 3) Web Member plane or vertical plane perpendicular to trusses.

1) Top Chord Plane

Most important to the builder or erection contractor is bracing in the plane of the top chord. Truss top chords are susceptible to lateral buckling before they are braced or sheathed.

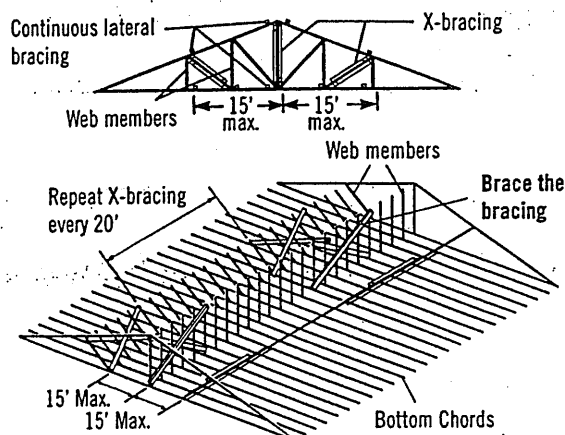


Exact spacing between trusses should be maintained as bracing is installed to avoid the hazardous practice of removing bracing to adjust spacing. This act of "adjusting spacing" can cause trusses to topple if connections are removed at the wrong time.

3) Web Member Plane

"X" bracing, as shown, is critical in preventing trusses from leaning or dominoing. Repeat as shown to create a succession of rigid units.

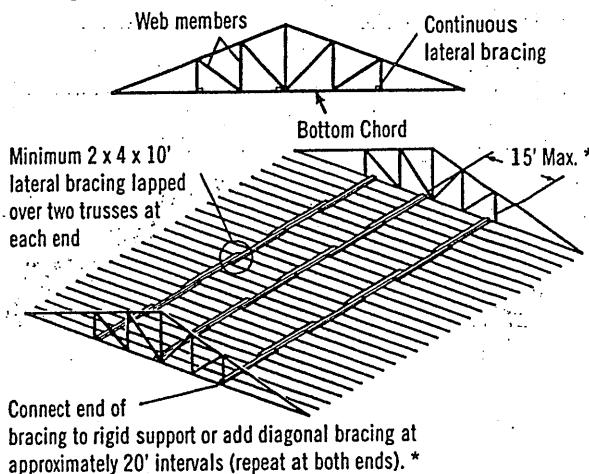
X-bracing should be installed on vertical web members wherever possible, at or near lateral bracing. Plywood or OSB may be substituted for X-bracing.



Note: Top chords and some web members are not shown, in order to make drawings more readable.

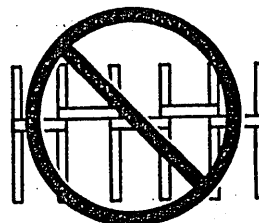
2) Bottom Chord Plane

In order to hold proper spacing on the bottom chord, temporary bracing is recommended on the top of the bottom chord.

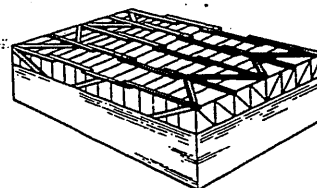


* Long spans, heavy loads or other spacing configurations may require closer spacing between lateral bracing and closer intervals between diagonals. Consult the building designer or HIB and DSB (Recommended Design Specification for Temporary Bracing of Metal-Plate Connected Wood Trusses) for details.

Diagonal or cross-bracing is very important!



Do not use short blocks to brace individual trusses without a specific bracing plan detailing their use

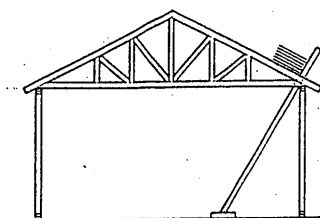
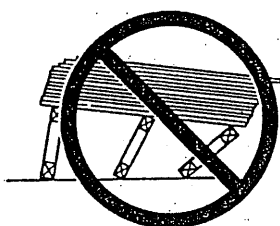


Bracing requirements using the same principles apply to parallel chord trusses

8 Stacking Materials

Do not proceed with building completion until all bracing is securely and properly in place

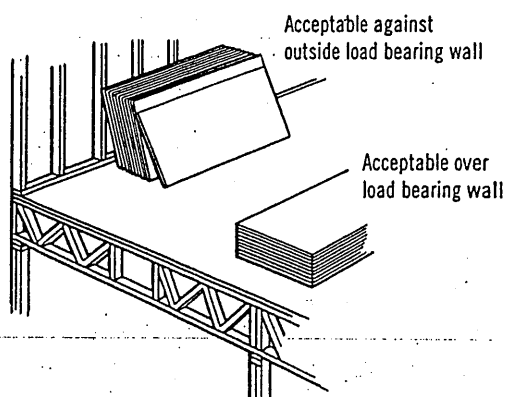
Never stack materials on unbraced or inadequately braced trusses



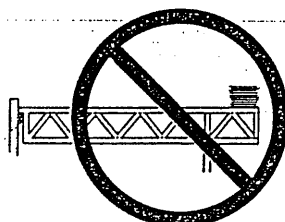
Platform must be rigidly braced

Proper distribution of construction materials is a must during construction.

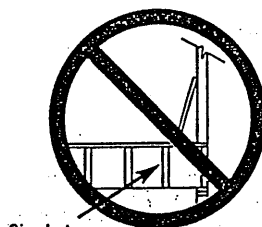
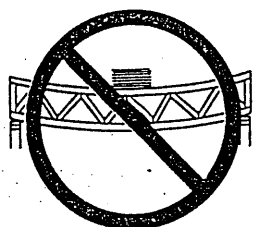
Never stack materials near a peak



Never stack materials on the cantilever of a truss

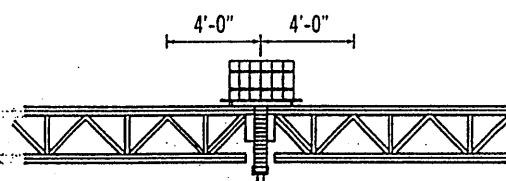


Always stack materials over two or more trusses.



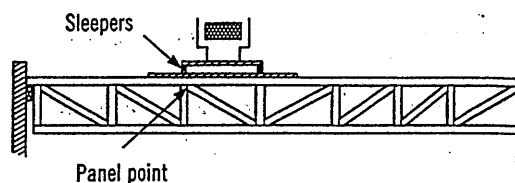
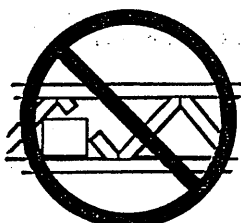
Single truss

Never overload small groups or single trusses. Position load over as many trusses as possible.



Roofing and mechanical contractors are cautioned to stack materials only along outside supporting members or directly over inside supporting members. Trusses are not designed for dynamic loads (i.e., moving vehicles). Extreme care should be taken when loading and stacking construction materials (rolled roofing, mechanical equipment, etc.) on the roof or floor system.

Never cut any structural member of a truss.



Sleepers

Panel point

Sleepers for mechanical equipment should be located at panel points (joints) or over main supporting members, and only on trusses that have been designed for such loads.

Caution Notes

Errors in building lines and/or dimensions, or errors by others shall be corrected by the contractor or responsible construction trade subcontractor or supplier before erection of trusses begins.

Cutting of nonstructural overhangs is considered a part of normal erection and shall be done by the builder or erection contractor.

Any field modification that involves the cutting, drilling, or relocation of any structural truss member or connector plate shall not be done without the approval of the truss manufacturer or a licensed design professional.

The methods and procedures outlined are intended to ensure that the overall construction techniques employed will put floor and roof trusses safely in place in a completed structure. These recommendations for bracing wood trusses originate from the collective experience of leading technical personnel in the wood truss industry, but must, due to the nature of responsibilities involved, be presented only as a guide for use by a qualified building designer, builder, or erection contractor. Thus, the Wood Truss Council of America expressly disclaims any responsibility for damages arising from the use, application, or reliance on the recommendations and information contained herein.

[Selected text and figures referenced or reproduced from HIB and DSB by permission of the Truss Plate Institute, Madison, WI.]

THE CANADIAN WOOD TRUSS ASSOCIATION



L'ASSOCIATION CANADIENNE DES FABRICANTS DE FERMES DE BOIS

1400 Blair Place, Suite 210, Ottawa, ON K1J 9B8
Tel.: 613-747-5544 Fax: 613-747-6264



Wood Truss Council of America

One WTCA Center
6300 Enterprise Lane, Madison, WI 53719-1140
Tel.: 608-274-4849 Fax: 608-274-3329
wtca@woodtruss.com www.woodtruss.com