

EWP DESIGN INC.

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RESPONSIBILITIES AND SPECIFICATIONS

RESPONSIBILITIES

1. EWP DESIGN INC. is responsible for the design of trusses as individual components.
2. It is the responsibility of others to ascertain that the design loads utilized on each drawing meet or exceed the actual dead load imposed by the structure, the live load imposed by the intended use and the snow load imposed by local building code or authorities with jurisdictions.
3. All dimensions are to be verified by the owner, contractor, architect or other authorities with jurisdictions before truss fabrication.
4. EWP DESIGN INC. bears no responsibility for the erection of trusses. Persons erecting trusses are cautioned to seek professional advice regarding the temporary and permanent bracing for the system. Bracing shown on EWP DESIGN INC. drawing is specified for the truss as a component only and forms an integral part of the truss design.
5. It is the truss manufacturer's responsibility to ensure that trusses are manufactured in conformance with specifications of EWP DESIGN INC. as outlined below.

SPECIFICATIONS

1. Trusses designed by EWP DESIGN INC. conform to the relevant section of the Ontario Building Code of Canada (Part 9 or Part 4) or to the Canadian code for farm buildings, whichever applies to the building type, as indicated on the EWP DESIGN INC. drawings, and conform to the design procedures established by the Truss Plate Institute of Canada. Unit stresses used for truss designs are as per the edition of CSA-O86 shown on EWP DESIGN INC. drawings.
2. Lumber is to be the size, species and grade as specified on EWP DESIGN INC. drawings.
3. Moisture content of lumber shall not exceed 19% in service unless specified otherwise.
4. Metal connector plates shall be applied to both faces of truss at each joint and shall be positioned as specified.
5. Top chords of trusses are assumed to be continuously braced laterally by roof sheathing or by purlins at intervals not exceeding 12.5 times the thickness of top chord member.
6. Bottom chords shall be laterally braced at intervals not exceeding 3M (10') o.c., where rigid ceiling is not applied directly to the underside of chords.

THESE DRAWINGS CONSTITUTE THE PROPERTY OF EWP DESIGN INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY THE ALPA LUMBER GROUP, AND WILL BE RETRACTED BY EWP DESIGN INC. IF UTILIZED FOR ANY OTHER PURPOSE.

[illegible]

LUMBER

N. L. G. A. RULES

CHORDS

SIZE

LUMBER

DESCR.

A - D

2x4

DRY

No.2

SPF

D - G

2x4

DRY

1650F 1.5E

SPF

G - J

2x4

DRY

1650F 1.5E

SPF

J - M

2x4

DRY

No.2

SPF

AA- B

2x6

DRY

No.2

SPF

N - M

2x4

DRY

No.2

SPF

AA- W

2x4

DRY

No.2

SPF

W - U

2x4

DRY

No.2

SPF

U - H

2x4

DRY

No.2

SPF

T - P

2x4

DRY

1650F 1.5E

SPF

O - L

2x4

DRY

No.2

SPF

O - N

2x4

DRY

No.2

SPF

ALL WEBS

2x3

DRY

No.2

SPF

EXCEPT

V - T

2x4

DRY

No.2

SPF

P - M

2x4

DRY

No.2

SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT

TYPE

PLATES

W

LEN

Y

X

B

TMVW-t

MT20

6.0

10.0

2.25

5.00

C

E, F, K

C

TMWWW-t

MT20

4.0

6.0

D

TTVWV-h

MT20

8.0

9.0

2.00

5.75

G

TS-t

MT20

3.0

8.0

H

TMV+p

MT20

2.0

4.0

I

TMWWW-t

MT20

4.0

6.0

1.75

3.00

J

TTVWV-h

MT20

8.0

9.0

2.00

5.75

L

TMVW-t

MT20

4.0

6.0

M

TMVW+p

MT20

6.0

7.0

Edge

2.50

N

BMVW1+t

MT20

5.0

6.0

Edge

1.50

O

BMV+p

MT20

2.0

4.0

P

BVMWW-I

MT20

6.0

10.0

3.25

6.75

Q, R, Y

Q

BMWWW-t

MT20

4.0

6.0

S

BMWWW+t

MT20

4.0

6.0

2.50

1.50

T

BVMWWW-I

MT20

6.0

12.0

Edge

4.00

U

BMV+p

MT20

2.0

4.0

V

BMWWW-t

MT20

8.0

9.0

Edge

2.25

W

BS-t

MT18HS

3.25

12.0

X

BMWWW+t

MT20

4.0

6.0

2.75

1.50

Z

BMWW-t

MT20

4.0

10.0

2.00

4.25

AA

BMV1+p

MT20

4.0

6.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED

GROSS REACTION

MAXIMUM FACTORED

INPUT

REQRD

JT

VERT

HORZ

DOWN

HORZ

UPLIFT

IN-SX

IN-SX

AA

3344

0

3344

0

0

5-8

5-6

N

3302

0

3302

0

0

MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT N. MINIMUM BEARING LENGTH AT JOINT N = 3-8.

ALLOW FOR 0.3" OF HORIZONTAL MOVEMENT DUE TO TOTAL LOAD

UNFACTORED REACTIONS

MAX./MIN. COMPONENT REACTIONS

JT

COMBINED

SNOW

LIVE

PERM.LIVE

WIND

DEAD

SOIL

AA

2384

1455 / 0

0 / 0

0 / 0

0 / 0

929 / 0

0 / 0

N

2355

1431 / 0

0 / 0

0 / 0

0 / 0

924 / 0

0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) AA

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.84 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT E-X

2x4 DRY SPF No.2 T-BRACE AT F-V

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

C H O R D S

W E B S

MEMB.

MAX. FACTORED FORCE (LBS)

FACTORED VERT. LOAD (PLF)

MAX. VERT. LOAD LC1 (LC)

MEMB.

MAX. FACTORED FORCE (LBS)

MAX. VERT. LOAD LC1 (LC)

FR-TO

FROM TO

LENGTH

FR-TO

A- B

0 / 35

-78.0

-78.0 0.16 (1)

10.00

Z- C

-809 / 0

0.24 (1)

B- C

-3750 / 0

-78.0

-78.0 0.47 (1)

3.24

C- Y

0 / 351

0.09 (1)

C- D

-4093 / 0

-78.0

-78.0 0.51 (1)

3.06

Y- D

-90 / 90

0.08 (1)

D- E

-4605 / 0

-134.0

-134.0 0.71 (1)

3.12

D- X

0 / 2006

0.50 (1)

E- F

-5117 / 0

-134.0

-134.0 0.76 (1)

2.92

X- E

-1436 / 0

0.98 (1)

F- G

-5834 / 0

-134.0

-134.0 0.71 (1)

2.84

E- V

0 / 848

0.21 (1)

G- H

-5834 / 0

-134.0

-134.0 0.71 (1)

2.84

V- F

-1487 / 0

0.53 (1)

H- I

-5855 / 0

-78.0

-78.0 0.53 (1)

3.03

V- T

0 / 5112

0.90 (1)

I- J

-5099 / 0

-78.0

-78.0 0.40 (1)

3.34

F- T

0 / 1066

0.26 (1)

J- K

-4484 / 0

-78.0

-78.0 0.40 (1)

3.00

T- I

0 / 1195

0.30 (1)

K- L

-4616 / 0

-78.0

-78.0 0.44 (1)

2.93

S- I

-1286 / 0

0.74 (1)

L- M

-3934 / 0

-78.0

-78.0 0.22 (1)

3.36

S- J

0 / 2219

0.55 (1)

AA- B

-3283 / 0

0.0

0.0 0.24 (1)

5.81

R- J

0 / 489

0.12 (1)

N- M

-3191 / 0

0.0

0.0 0.38 (1)

4.76

R- K

-207 / 0

0.08 (1)

AA- Z

0 / 0

-31.8

-31.8 0.13 (4)

10.00

Q- K

-70 / 21

0.02 (1)

Z- Y

0 / 3135

-31.8

-31.8 0.65 (1)

10.00

Q- L

0 / 529

0.13 (1)

Y- X

0 / 3383

-31.8

-31.8 0.69 (1)

10.00

B- Z

0 / 3271

0.81 (1)

X- W

0 / 4605

-31.8

-31.8 0.92 (1)

10.00

P- N

-133 / 0

0.02 (1)

W- V

0 / 4605

-31.8

-31.8 0.92 (1)

10.00

P- M

0 / 3407

0.60 (1)

V- U

0 / 117

-31.8

-31.8 0.22 (4)

10.00

U- T

0 / 72

0.0

0.0 0.19 (1)

10.00

T- H

-506 / 0

0.0

0.0 0.34 (1)

7.81

T- S

0 / 5099

-87.8

-87.8 0.88 (1)

10.00

S- R

0 / 3721

-87.8

-87.8 0.70 (1)

10.00

R- Q

0 / 3852

-87.8

-87.8 0.63 (1)

10.00

Q- P

0 / 3366

-87.8

-87.8 0.55 (1)

10.00

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***

GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.

LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP

CH.

LL

=

21.0

PSF

DL

=

6.0

PSF

BOT

CH.

LL

=

0.0

PSF

DL

=

7.4

PSF

TOTAL

LOAD

=

34.4

PSF

SPACING =

24.0

IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip

LEFT SETBACK = 8-1-12

RIGHT SETBACK = 7-8-13

END SETBACK = 4-10-8

END WALL WIDTH = 0-0

CORNER FRAMING TYPE: CONVENTIONAL

END JACK TYPE: CONVENTIONAL

APPLIED TO FRONT SIDE

- ADDTL LOADS BASED ON 55 % OF GSL.

LOADS APPLIED TO FIRST 23-7-0 OF SPAN MEASURED FROM THE LEFT.

GIRDER TYPE: CStdGirder

START DISTANCE = 23-7-0

START SPAN CARRIED = 4-10-8

END DISTANCE = 38-11-0

END SPAN CARRIED = 4-10-8

END WALL WIDTH = 0-0

APPLIED TO FRONT SIDE OF BOTTOM CHORD.

- ADDTL LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019

- PART 9 OF OBC 2012 (2019 AMENDMENT)

- CSA 086-14

- TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.34")

CALCULATED VERT. DEFL.(LL)= L/ 999 (0.32")

ALLOWABLE DEFL.(TL)= L/360 (1.34")

CALCULATED VERT. DEFL.(TL)= L/ 769 (0.63")

CSI: TC=0.76/1.00 (E-F:1) , BC=0.92/1.00 (V-X:1) , WB=0.98/1.00 (E-X:1) , SSI=0.35/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

LATERAL BRACE(S) SHOWN SHALL BE
2X4 SPF#2



O- P 0 / 24 0.0 0.0 0.17 (1) 10.00

P- L -739 / 0 0.0 0.0 0.18 (1) 7.81

O- N 0 / 98 -33.5 -33.5 0.03 (4) 10.00

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	8-1-12	-357	-357	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

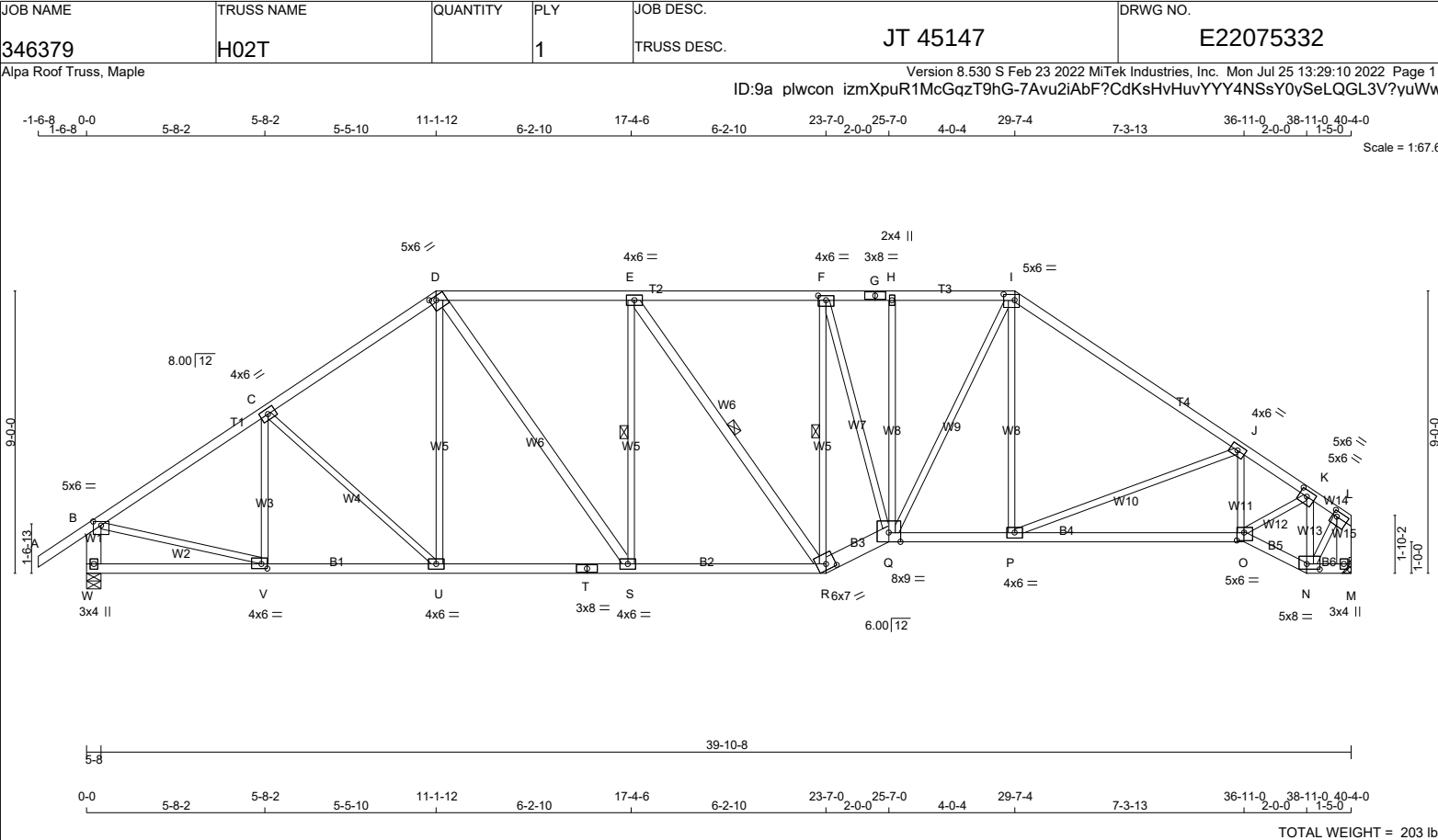
PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
MT18HS	586	403	2455
		1382	3163
			3004

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (T) (INPUT = 0.90)

JSI METAL= 0.85 (W) (INPUT = 1.00)



N. L. G. A. RULES										DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER										DESIGN CRITERIA									
CHORDS SIZE LUMBER DESCR.										BEARINGS										SPECIFIED LOADS:									
A - D 2x4 DRY No.2 SPF										FACTORED MAXIMUM FACTORED INPUT REQRD										TOP CH. LL = 21.0 PSF									
D - G 2x4 DRY No.2 SPF										GROSS REACTION GROSS REACTION BRG BRG										DL = 6.0 PSF									
G - I 2x4 DRY No.2 SPF										JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX										BOT CH. LL = 0.0 PSF									
I - L 2x4 DRY No.2 SPF										W 2072 0 2072 0 0 5-8 2-4										DL = 7.4 PSF									
W - B 2x6 DRY No.2 SPF										M 1945 0 1945 0 0 MECHANICAL										TOTAL LOAD = 34.4 PSF									
M - L 2x6 DRY No.2 SPF										A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT M. MINIMUM BEARING LENGTH AT JOINT M = 2-2.										SPACING = 24.0 IN. C/C									
W - T 2x4 DRY No.2 SPF										UNFACTORED REACTIONS										LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM									
T - R 2x4 DRY No.2 SPF										1ST LCASE MAX./MIN. COMPONENT REACTIONS										THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015									
R - Q 2x6 DRY No.2 SPF										JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL										THIS DESIGN COMPLIES WITH:									
Q - O 2x4 DRY No.2 SPF										W 1475 915 / 0 0 / 0 0 / 0 560 / 0 0 / 0										- PART 9 OF BCBC 2018 , ABC 2019									
O - N 2x4 DRY No.2 SPF										M 1387 847 / 0 0 / 0 0 / 0 540 / 0 0 / 0										- PART 9 OF OBC 2012 (2019 AMENDMENT)									
N - M 2x4 DRY No.2 SPF										BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W										- CSA 086-14									
ALL WEBS 2x3 DRY No.2 SPF										BRACING										- TPIC 2014									
EXCEPT										TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.74 FT.										THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015									
D - S 2x4 DRY No.2 SPF										MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.										THIS DESIGN COMPLIES WITH:									
E - R 2x4 DRY No.2 SPF										ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.										- PART 9 OF BCBC 2018 , ABC 2019									
DRY: SEASONED LUMBER.										1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-S, E-R, F-R.										- PART 9 OF OBC 2012 (2019 AMENDMENT)									
										END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW										- CSA 086-14									
										LOADING										- TPIC 2014									
										TOTAL LOAD CASES: (4)										(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD									
PLATES (table is in inches)																				ALLOWABLE DEFL.(LL)= L/360 (1.34")									
JT TYPE PLATES W LEN Y X																				CALCULATED VERT. DEFL.(LL) = L/999 (0.13")									
B TMVW-p MT20 5.0 6.0 1.50 3.00																				ALLOWABLE DEFL.(TL)= L/360 (1.34")									
C, E, J																				CALCULATED VERT. DEFL.(TL) = L/999 (0.27")									
C TMWW-t MT20 4.0 6.0																				CSI: TC=0.72/1.00 (I-J:1) , BC=0.50/1.00 (O-P:1) , WB=0.51/1.00 (F-R:1) , SSI=0.23/1.00 (D-E:1)									
D TTWW-h MT20 5.0 6.0 1.50 2.25																				DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10									
F TMWW-t MT20 4.0 6.0 1.75 3.00																				COMPANION LIVE LOAD FACTOR = 1.00									
G TS-t MT20 3.0 8.0																				AUTOSOLVE HEELS OFF									
H TMW+w MT20 2.0 4.0																				TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .									
I TTWW-I MT20 5.0 6.0 2.25 4.25																				NAIL VALUES									
K TMWW-t MT20 5.0 6.0 2.25 3.00																				PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)									
L TMVW-t MT20 5.0 6.0 2.00 1.75																				MAX MIN MAX MIN MAX MIN									
M BMV1+p MT20 3.0 4.0																				MT20 650 371 1747 788 1987 1873									
N BBWW-I MT20 5.0 8.0 2.00 5.00																				PLATE PLACEMENT TOL. = 0.250 inches									
O BBWW-I MT20 5.0 6.0 3.00 2.75																				PLATE ROTATION TOL. = 5.0 Deg.									
P, S, U																				JSI GRIP= 0.90 (D) (INPUT = 0.90)									
P BMWW-t MT20 4.0 6.0																				JSI METAL= 0.52 (T) (INPUT = 1.00)									
Q BBWWV-p MT20 8.0 9.0 Edge																													
R BBWW-h MT20 6.0 7.0 2.25 3.50																													
T BS-t MT20 3.0 8.0																													
V BMWW-t MT20 4.0 6.0 1.75 2.25																													
W BMV1+p MT20 3.0 4.0																													
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.																													

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

Y. WIDYA
100225448
07/25/2022
PROVINCE OF ONTARIO

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
346379	H03T		1	TRUSS DESC.	E22075334

Alpa Roof Truss, Maple

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ID:9a plwcon izmXpuR1McGqzT9hG-0x9Ou4D5IEi3oUbgWk UiOE2XTvJuHxzLuJHemyuWw

Scale = 1:69.9

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
I - J	2x4	DRY	No.2	SPF
J - O	2x4	DRY	No.2	SPF
AA- B	2x6	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
AA- X	2x6	DRY	No.2	SPF
X - V	2x6	DRY	No.2	SPF
V - U	2x6	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
R - Q	2x4	DRY	No.2	SPF
Q - P	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
Y - E	2x4	DRY	No.2	SPF
E - W	2x4	DRY	No.2	SPF
W - F	2x4	DRY	No.2	SPF
F - V	2x4	DRY	No.2	SPF
V - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	10.0	1.50	5.00
C	F, G, K, L					
C	TMVW-t	MT20	4.0	6.0		
D	TS-t	MT20	3.0	8.0		
E	TTWW-I	MT20	4.0	10.0	2.00	7.75
H	TMW+w	MT20	2.0	4.0		
I	TTWW-h	MT20	5.0	6.0	1.50	2.75
J	TS-t	MT20	3.0	8.0		
M	TMVW-t	MT20	4.0	6.0	1.50	3.00
N	TMVW-t	MT20	5.0	6.0	2.00	1.75
P	BMV1+p	MT20	3.0	4.0		
Q	BBWW-I	MT20	5.0	8.0	2.50	6.00
R	BBWW-I	MT20	5.0	6.0	3.00	3.00
S, T, W, Y						
S	BMVW-t	MT20	4.0	6.0		
U	BBWWW-p	MT20	8.0	9.0	Edge	
V	BBWW-I	MT20	6.0	10.0	4.50	8.00
X	BS-t	MT20	4.0	6.0		
Z	BMVW-t	MT20	4.0	6.0	2.00	1.50
AA	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
JT	2183	0	2183	0	0	5-8	2-6
AA	2188	0	2188	0	0	5-8	2-6

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
AA	1562	922 / 0	0 / 0	0 / 0	0 / 0	640 / 0	0 / 0
P	1565	925 / 0	0 / 0	0 / 0	0 / 0	641 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) AA, P

BRACING

FOR SECTION E-I, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.61 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-Y, F-W, F-V, G-V, K-T.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBBS				
MEMB.	FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC (LBS)	MEMB.	FACTORED FORCE (LBS)	MAX CSI (LC)		
FR-TO		FROM	TO	LENGTH	FR-TO				
A-B	0 / 35	-78.0	-78.0 0.15 (1)	10.00	Z-C	-213 / 16	0.13 (1)		
B-C	-2548 / 0	-78.0	-78.0 0.78 (1)	3.61	C-Y	-414 / 0	0.24 (1)		
C-D	-2243 / 0	-78.0	-78.0 0.69 (1)	3.87	Y-E	0 / 384	0.06 (1)		
D-E	-2243 / 0	-78.0	-78.0 0.69 (1)	3.87	E-W	0 / 502	0.08 (1)		
E-F	-2042 / 0	-93.0	-93.0 0.28 (1)	2.00	W-F	-382 / 0	0.23 (1)		
F-AB	-1993 / 0	-93.0	-93.0 0.28 (1)	2.00	F-V	-121 / 0	0.09 (1)		
AB-G	-1993 / 0	-93.0	-93.0 0.28 (1)	2.00	V-G	-776 / 0	0.46 (1)		
G-H	-2070 / 0	-93.0	-93.0 0.17 (1)	2.00	G-U	0 / 405	0.09 (1)		
H-I	-2071 / 0	-93.0	-93.0 0.12 (1)	2.00	U-H	0 / 50	0.01 (1)		
I-J	-2433 / 0	-78.0	-78.0 0.35 (1)	4.13	U-I	0 / 606	0.14 (1)		
J-K	-2433 / 0	-78.0	-78.0 0.35 (1)	4.13	T-I	0 / 424	0.10 (1)		
K-L	-2834 / 0	-78.0	-78.0 0.38 (1)	3.85	T-K	-572 / 0	0.26 (1)		
L-M	-3050 / 0	-78.0	-78.0 0.20 (1)	3.86	S-K	0 / 210	0.05 (4)		
M-N	-1742 / 0	-78.0	-78.0 0.14 (1)	4.94	S-L	-227 / 0	0.13 (1)		
N-O	0 / 35	-78.0	-78.0 0.15 (1)	10.00	R-L	-74 / 16	0.01 (1)		
AA-B	-2122 / 0	0.0	0.0 0.14 (1)	6.97	R-M	0 / 1466	0.33 (1)		
P-N	-2171 / 0	0.0	0.0 0.15 (1)	6.91	Q-M	-1590 / 0	0.26 (1)		
					B-Z	0 / 2179	0.49 (1)		
					Q-N	0 / 1703	0.38 (1)		
AA-Z	0 / 0	-18.5	-18.5 0.10 (4)	10.00					
Z-Y	0 / 2151	-18.5	-18.5 0.30 (1)	10.00					
Y-X	0 / 1838	-18.5	-18.5 0.26 (1)	10.00					
X-W	0 / 1838	-18.5	-18.5 0.26 (1)	10.00					
W-V	0 / 2042	-18.5	-18.5 0.27 (1)	10.00					
V-U	0 / 2198	-18.5	-18.5 0.27 (1)	10.00					
U-T	0 / 2007	-18.5	-18.5 0.38 (1)	10.00					
T-S	0 / 2370	-18.5	-18.5 0.44 (1)	10.00					
S-R	0 / 2568	-18.5	-18.5 0.47 (1)	10.00					
R-Q	0 / 1501	-18.5	-18.5 0.25 (1)	10.00					
Q-P	0 / 0	-18.5	-18.5 0.02 (4)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	21.0	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON
PIGGYBACK TRUSS WITH SLOPES OF 6.00/12
AND -6.00/12 AND RESPECTIVE HEEL HEIGHTS
OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD
LOAD OF 6.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= $L/360$ (1.36")
 CALCULATED VERT. DEFL.(LL) = $L/999$ (0.11")
 ALLOWABLE DEFL.(TL)= $L/360$ (1.36")
 CALCULATED VERT. DEFL.(TL) = $L/999$ (0.24")

CSI: TC=0.78/1.00 (B-C:1), BC=0.47/1.00 (R-S:1),
WB=0.49/1.00 (B-Z:1), SSI=0.22/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES						
PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (N) (INPUT = 0.90)
JSI METAL= 0.49 (Z) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE
2X4 SPF#2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
346379	H03TS		1	JT 45147	E22075335

Alpa Roof Truss, Maple

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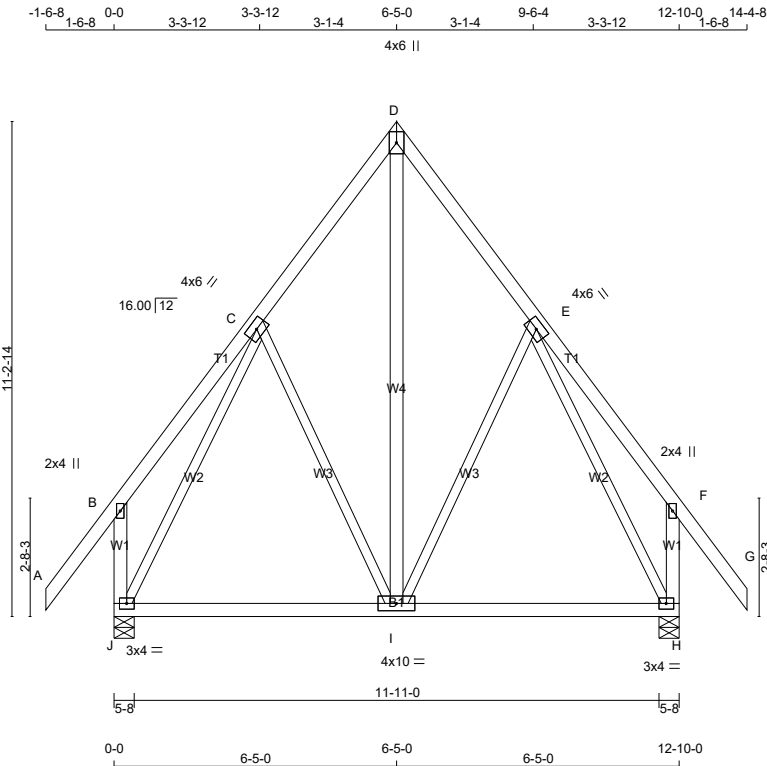
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TOTAL WEIGHT = 2 X 255 = 509

[illegible]

LATERAL BRACE(S) SHOWN SHALL BE
2X4 SPF#2





TOTAL WEIGHT = 2 X 86 = 171 lb

[M][F]

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
I - D	2x4	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	2.0	4.0	
C	TMWW-t	MT20	4.0	6.0	
D	TTW+p	MT20	4.0	6.0	
E	TMWW-t	MT20	4.0	6.0	
F	TMV+p	MT20	2.0	4.0	
H	BMVW1-t	MT20	3.0	4.0	
I	BMWWW-t	MT20	4.0	10.0	
J	BMVW1-t	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER							
BEARINGS							
		FACTORED		MAXIMUM FACTORED		INPUT	
		GROSS REACTION		GROSS REACTION		BRG	
JT		VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX
J		748	0	748	0	5-8	1-8
H		748	0	748	0	5-8	1-8

UNFACTORED REACTIONS								
		1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT		COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J		531	339 / 0	0 / 0	0 / 0	0 / 0	192 / 0	0 / 0
H		531	339 / 0	0 / 0	0 / 0	0 / 0	192 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

C H O R D S					W E B S				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX	CSI (LC)	
FR-TO		FROM	TO		FR-TO				
A-B	0 / 52	-78.0	-78.0	0.15 (1)	10.00	I-D	0 / 311	0.05 (1)	
B-C	0 / 24	-78.0	-78.0	0.13 (1)	10.00	I-E	-92 / 0	0.10 (1)	
C-D	-342 / 0	-78.0	-78.0	0.10 (1)	6.25	C-I	-92 / 0	0.10 (1)	
D-E	-342 / 0	-78.0	-78.0	0.10 (1)	6.25	J-C	-534 / 0	0.54 (1)	
E-F	0 / 24	-78.0	-78.0	0.13 (1)	10.00	E-H	-534 / 0	0.54 (1)	
F-G	0 / 52	-78.0	-78.0	0.15 (1)	10.00				
J-B	-222 / 0	0.0	0.0	0.03 (1)	7.81				
H-F	-222 / 0	0.0	0.0	0.03 (1)	7.81				
J-I	0 / 233	-18.5	-18.5	0.24 (4)	10.00				
I-H	0 / 233	-18.5	-18.5	0.24 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.15/1.00 (A-B:1) , BC=0.24/1.00 (H-I:4) ,
WB=0.54/1.00 (C-J:1) , SSI=0.10/1.00 (I-J:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.66 (J) (INPUT = 0.90)
JSI METAL= 0.13 (E) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

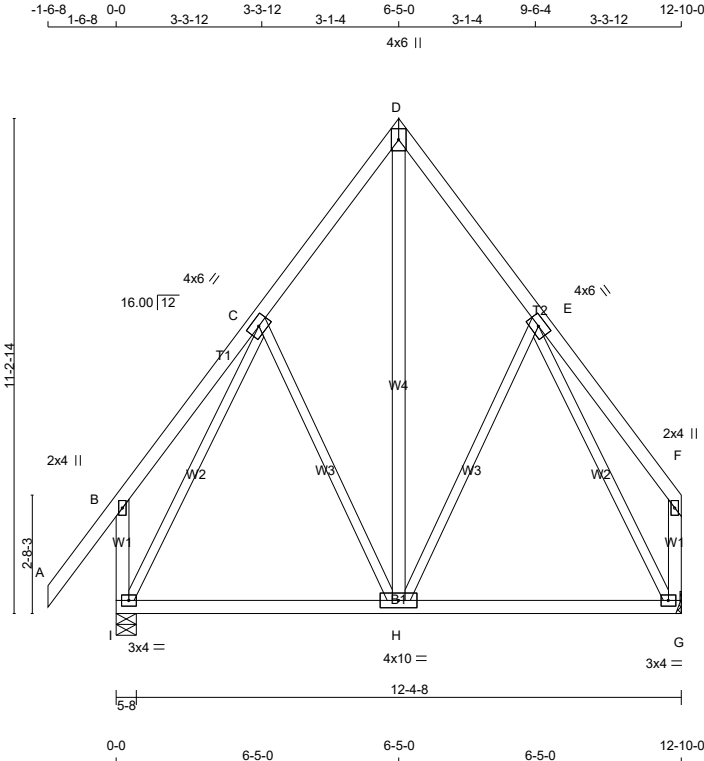


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
346379	H06S		1	TRUSS DESC. JT 45147	E22075339

Alpa Roof Truss, Maple

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Scale = 1:52.3

TOTAL WEIGHT = 83 lb

N. L. G. A. RULES										DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER										DESIGN CRITERIA										[M][F]
CHORDS SIZE LUMBER										BEARINGS										SPECIFIED LOADS:										
A - D 2x4 DRY No.2										FACTORED MAXIMUM FACTORED INPUT REQRD										TOP CH. LL = 21.0 PSF										
D - F 2x4 DRY No.2										GROSS REACTION GROSS REACTION BRG BRG										DL = 6.0 PSF										
I - B 2x4 DRY No.2										JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX										BOT CH. LL = 0.0 PSF										
G - F 2x4 DRY No.2										I 748 0 748 0 0 5-8 1-8										DL = 7.4 PSF										
I - G 2x4 DRY No.2										G 619 0 619 0 0 MECHANICAL										TOTAL LOAD = 34.4 PSF										
ALL WEBS 2x3 DRY No.2										A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 1-8.										SPACING = 24.0 IN. C/C										
EXCEPT																				THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015										
H - D 2x4 DRY No.2																				THIS DESIGN COMPLIES WITH:										
DRY: SEASONED LUMBER.																				- PART 9 OF CBC 2018 , ABC 2019										
																				- PART 9 OF OBC 2012 (2019 AMENDMENT)										
																				- CSA 086-14										
																				- TPIC 2014										
																				(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD										
																				ALLOWABLE DEFL.(LL)= L/360 (0.43")										
																				CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")										
																				ALLOWABLE DEFL.(TL)= L/360 (0.43")										
																				CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")										
																				CSI: TC=0.15/1.00 (A-B:1) , BC=0.24/1.00 (H-I:4) , WB=0.54/1.00 (C-I:1) , SSI=0.10/1.00 (H-I:4)										
																				DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10										
																				COMP=1.10 SHEAR=1.10 TENS= 1.10										
																				COMPANION LIVE LOAD FACTOR = 1.00										
																				TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .										
																				NAIL VALUES										
																				PLATE GRIP(DRY) SHEAR SECTION										
																				(PSI) (PLI) (PLI)										
																				MAX MIN MAX MIN MAX MIN										
																				MT20 650 371 1747 788 1987 1873										
																				PLATE PLACEMENT TOL. = 0.250 inches										
																				PLATE ROTATION TOL. = 5.0 Deg.										
																				JSI GRIP= 0.66 (G) (INPUT = 0.90)										
																				JSI METAL= 0.13 (C) (INPUT = 1.00)										

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

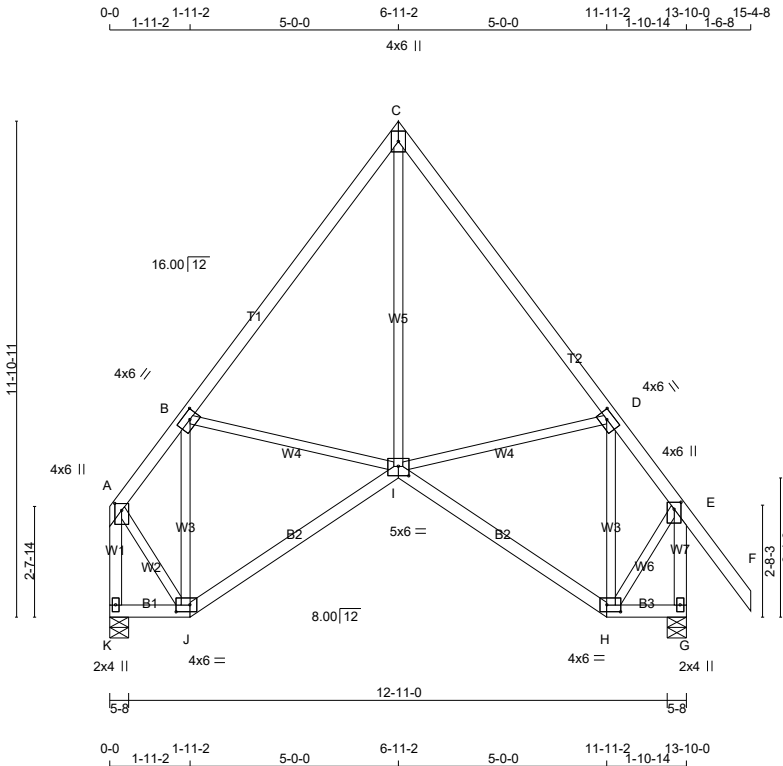


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
346379	H09T		1	TRUSS DESC. JT 45147	E22075344

Alpa Roof Truss, Maple

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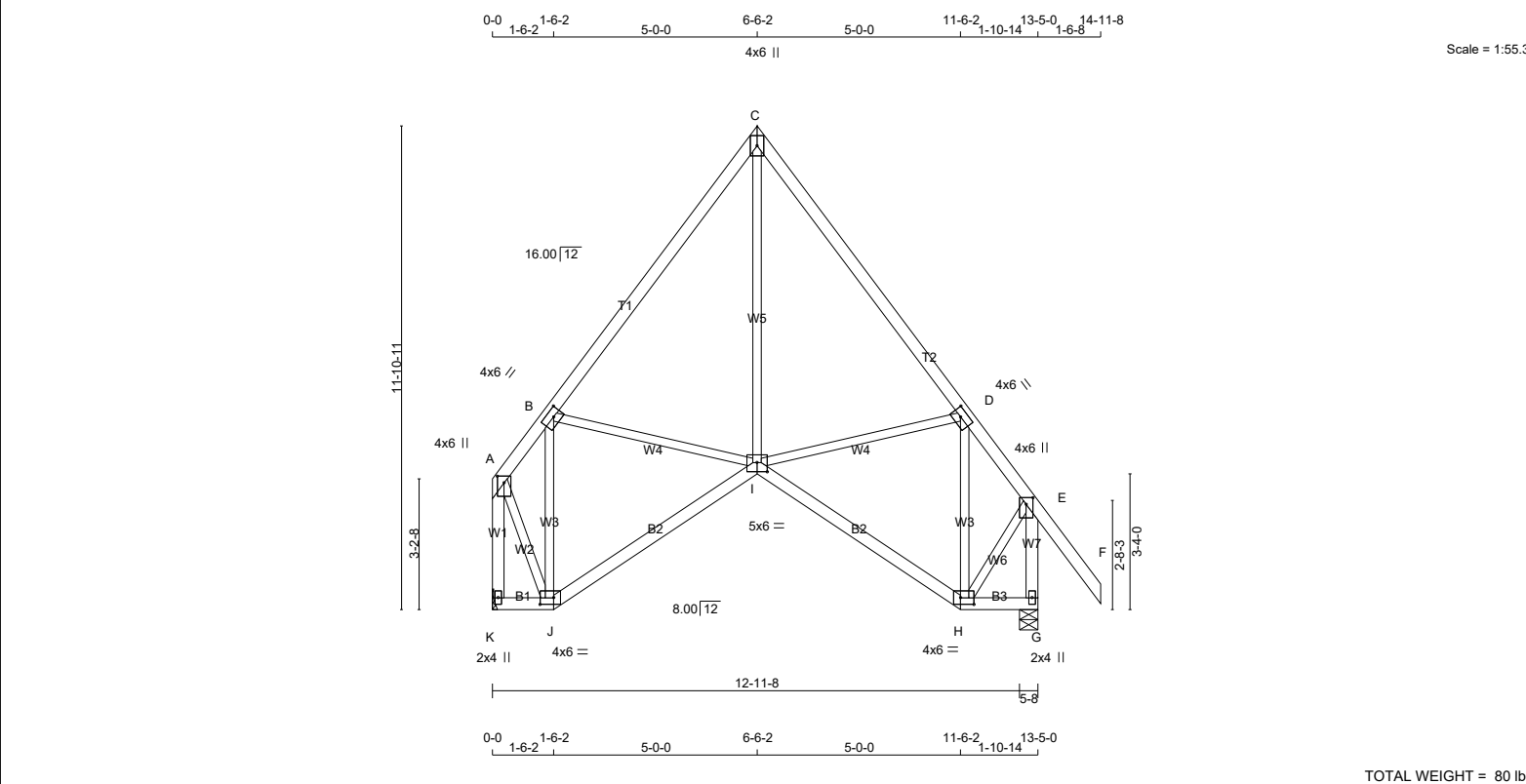


LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR.				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER								DESIGN CRITERIA				[M][F]
				BEARINGS												
				FACTORED		MAXIMUM FACTORED		INPUT		REQRD		SPECIFIED LOADS:				
				GROSS REACTION		GROSS REACTION		BRG		BRG		TOP CH. LL = 21.0 PSF				
				JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	DL = 6.0 PSF				
A - C 2x4 DRY No.2 SPF				K	659	0	659	0	0	5-8	1-8	BOT CH. LL = 0.0 PSF				
C - F 2x4 DRY No.2 SPF				G	804	0	804	0	0	5-8	1-8	DL = 7.4 PSF				
K - A 2x4 DRY No.2 SPF												TOTAL LOAD = 34.4 PSF				
G - E 2x4 DRY No.2 SPF																
K - J 2x4 DRY No.2 SPF																
J - I 2x4 DRY No.2 SPF																
I - H 2x4 DRY No.2 SPF																
H - G 2x4 DRY No.2 SPF																
				UNFACTORED REACTIONS								SPACING = 24.0 IN. C/C				
				1ST LCASE		MAX./MIN. COMPONENT REACTIONS										
ALL WEBS 2x3 DRY No.2 SPF				JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	THIS TRUSS IS DESIGNED FOR RESIDENTIAL				
EXCEPT				K	470	286 / 0	0 / 0	0 / 0	0 / 0	184 / 0	0 / 0	OR SMALL BUILDING REQUIREMENTS OF PART				
				G	571	364 / 0	0 / 0	0 / 0	0 / 0	206 / 0	0 / 0	9, NBCC 2015				
DRY: SEASONED LUMBER.				BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, G								THIS DESIGN COMPLIES WITH:				

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.00	2.00
B	TMWW-t	MT20	4.0	6.0	2.00	2.50
C	TTW+p	MT20	4.0	6.0		
D	TMWW-t	MT20	4.0	6.0	2.00	2.50
E	TMVW+p	MT20	4.0	6.0	2.00	2.00
G	BMV1+p	MT20	2.0	4.0		
H	BBWW-I	MT20	4.0	6.0	2.00	4.00
I	BBWWW-p	MT20	5.0	6.0	2.75	3.00
J	BBWW-I	MT20	4.0	6.0	2.00	4.00
K	BMV1+p	MT20	2.0	4.0		

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2





LUMBER					TOTAL WEIGHT = 80 lb				
N. L. G. A. RULES					[M][F]				
CHORDS	SIZE		LUMBER	DESCR.					
A - C	2x4	DRY	No.2	SPF					
C - F	2x4	DRY	No.2	SPF					
K - A	2x4	DRY	No.2	SPF					
G - E	2x4	DRY	No.2	SPF					
K - J	2x4	DRY	No.2	SPF					
J - I	2x4	DRY	No.2	SPF					
I - H	2x4	DRY	No.2	SPF					
H - G	2x4	DRY	No.2	SPF					
ALL WEBS	2x3	DRY	No.2	SPF					
EXCEPT									
DRY: SEASONED LUMBER.									

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.00	2.00
B	TMWW-t	MT20	4.0	6.0	2.00	2.50
C	TTW+p	MT20	4.0	6.0		
D	TMWW-t	MT20	4.0	6.0	2.00	2.50
E	TMVW+p	MT20	4.0	6.0	2.00	2.00
G	BMV1+p	MT20	2.0	4.0		
H	BBWW-I	MT20	4.0	6.0	2.00	4.00
I	BBWWW-p	MT20	5.0	6.0	2.75	3.00
J	BBWW-I	MT20	4.0	6.0	2.00	4.00
K	BMV1+p	MT20	2.0	4.0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G									
BRACING									
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.									
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.									
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.									
LOADING									
TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)		
FR-TO		FROM TO			FR-TO				
A- B	-259 / 0	-78.0	-78.0 0.20 (1)	6.25	J- B	-494 / 0	0.18 (1)		
B- C	-464 / 0	-78.0	-78.0 0.25 (1)	6.25	B- I	0 / 51	0.01 (1)		
C- D	-461 / 0	-78.0	-78.0 0.25 (1)	6.25	I- C	0 / 368	0.08 (1)		
D- E	-369 / 0	-78.0	-78.0 0.20 (1)	6.25	I- D	0 / 30	0.01 (1)		
E- F	0 / 52	-78.0	-78.0 0.15 (1)	10.00	H- D	-377 / 0	0.14 (1)		
K- A	-625 / 0	0.0	0.0 0.11 (1)	7.81	A- J	0 / 469	0.11 (1)		
G- E	-767 / 0	0.0	0.0 0.11 (1)	7.81	H- E	0 / 373	0.08 (1)		
K- J	0 / 0	-18.5	-18.5 0.01 (4)	10.00					
J- I	0 / 254	-18.5	-18.5 0.16 (4)	10.00					
I- H	0 / 278	-18.5	-18.5 0.16 (4)	10.00					
H- G	0 / 0	-18.5	-18.5 0.02 (4)	10.00					

DESIGN CRITERIA				
- TPIC 2014				
DESIGN ASSUMPTIONS				
-OVERHANG NOT TO BE ALTERED OR CUT OFF.				
(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD				
ALLOWABLE DEFL.(LL)= L/360 (0.45")				
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")				
ALLOWABLE DEFL.(TL)= L/360 (0.45")				
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")				
CSI: TC=0.25/1.00 (C-D:1) , BC=0.16/1.00 (H-I:4) , WB=0.18/1.00 (B-J:1) , SSI=0.11/1.00 (B-C:1)				
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10				
COMP=1.10 SHEAR=1.10 TENS= 1.10				
COMPANION LIVE LOAD FACTOR = 1.00				
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .				
NAIL VALUES				
PLATE	GRIP(DRY)	SHEAR	SECTION	
	(PSI)	(PLI)	(PLI)	
	MAX MIN	MAX MIN	MAX MIN	
MT20	650 371	1747 788	1987 1873	
PLATE PLACEMENT TOL. = 0.250 inches				
PLATE ROTATION TOL. = 5.0 Deg.				
JSI GRIP= 0.74 (J) (INPUT = 0.90)				
JSI METAL= 0.21 (J) (INPUT = 1.00)				

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

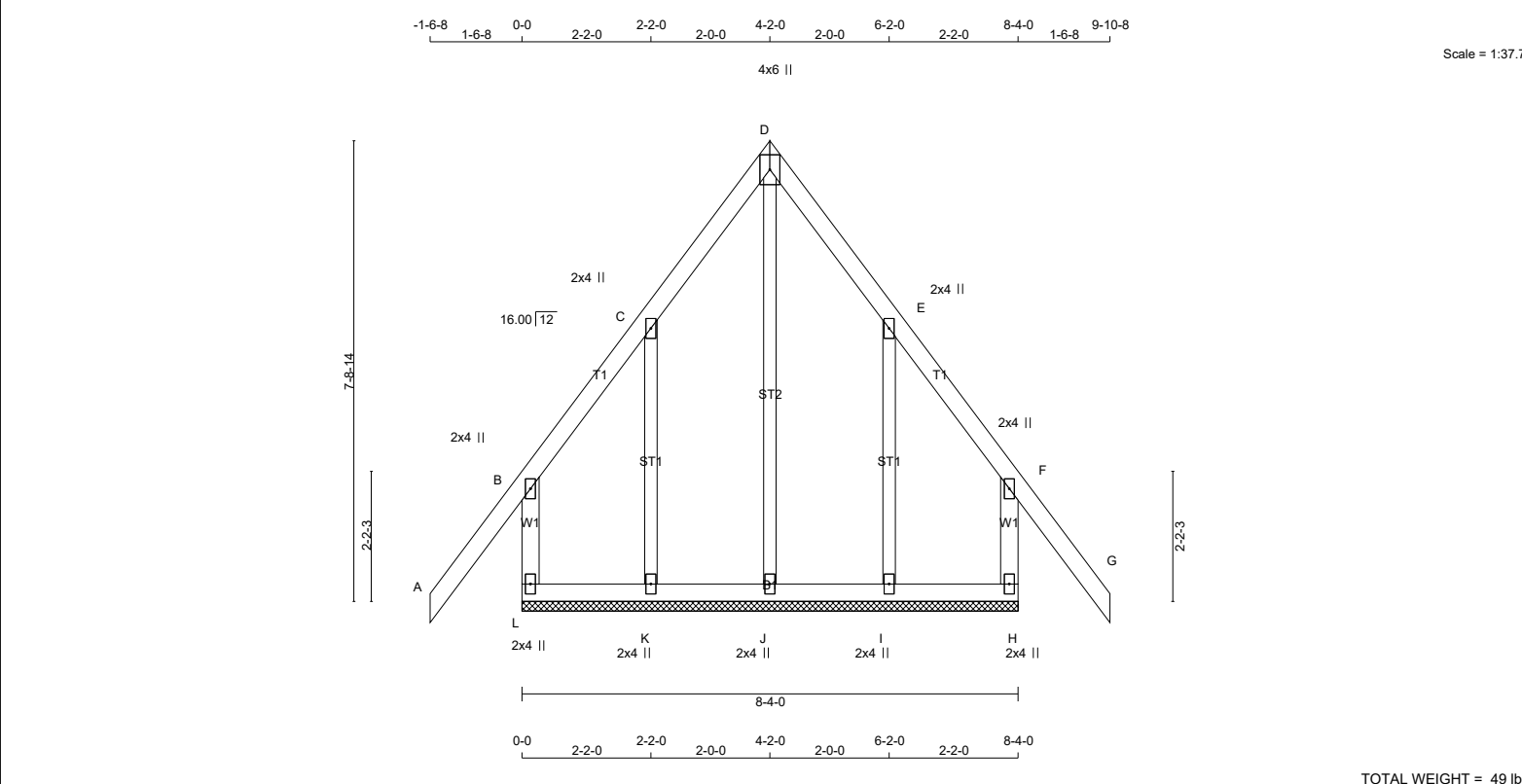


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
346379	H10G		1	TRUSS DESC. JT 45147	E22075347

Alpa Roof Truss, Maple

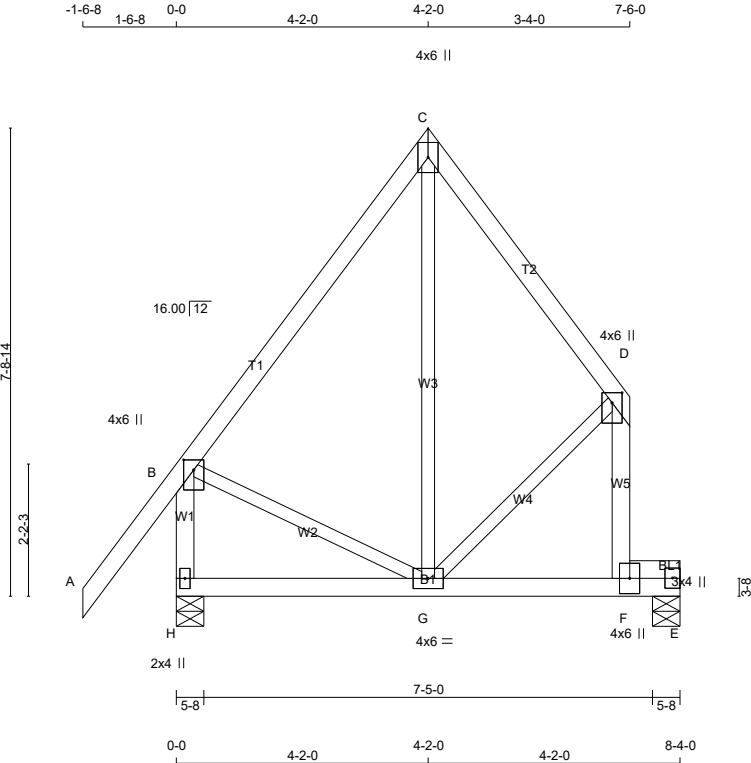
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ID:9a plwcon izmXpuR1McGqzT9hG-NAUiWbUv7?UxRsGvoLNeb19yiLXa1IGAPJeROUyuWVz



TOTAL WEIGHT = 49 lb

LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. A - D 2x4 DRY No.2 SPF D - G 2x4 DRY No.2 SPF L - B 2x4 DRY No.2 SPF H - F 2x4 DRY No.2 SPF L - H 2x4 DRY No.2 SPF ALL WEBS 2x3 DRY No.2 SPF ALL GABLE WEBS 2x3 DRY No.2 SPF DRY: SEASONED LUMBER. GABLE STUDS SPACED AT 2-0-0 OC.					DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS. THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE. BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (4) <table><tr><th colspan="4">CHORDS</th><th colspan="4">WEBS</th></tr><tr><th>MEMB.</th><th>MAX. FACTORED FORCE (LBS)</th><th>VERT. LOAD (PLF)</th><th>LC1 MAX (LC)</th><th>MAX. UNBRAC LENGTH</th><th>MEMB.</th><th>MAX. FACTORED FORCE (LBS)</th><th>MAX CS1 (LC)</th></tr><tr><td>FR-TO</td><td></td><td>FROM</td><td>TO</td><td></td><td>FR-TO</td><td></td><td></td></tr><tr><td>A- B</td><td>0 / 52</td><td>-78.0</td><td>-78.0</td><td>0.15 (1)</td><td>10.00</td><td>J- D</td><td>-237 / 0 0.24 (1)</td></tr><tr><td>B- C</td><td>0 / 16</td><td>-78.0</td><td>-78.0</td><td>0.09 (1)</td><td>10.00</td><td>K- C</td><td>-132 / 0 0.05 (1)</td></tr><tr><td>C- D</td><td>0 / 43</td><td>-78.0</td><td>-78.0</td><td>0.05 (1)</td><td>10.00</td><td>I- E</td><td>-132 / 0 0.05 (1)</td></tr><tr><td>D- E</td><td>0 / 43</td><td>-78.0</td><td>-78.0</td><td>0.05 (1)</td><td>10.00</td><td></td><td></td></tr><tr><td>E- F</td><td>0 / 16</td><td>-78.0</td><td>-78.0</td><td>0.09 (1)</td><td>10.00</td><td></td><td></td></tr><tr><td>F- G</td><td>0 / 52</td><td>-78.0</td><td>-78.0</td><td>0.15 (1)</td><td>10.00</td><td></td><td></td></tr><tr><td>L- B</td><td>-204 / 0</td><td>0.0</td><td>0.0</td><td>0.06 (1)</td><td>7.81</td><td></td><td></td></tr><tr><td>H- F</td><td>-204 / 0</td><td>0.0</td><td>0.0</td><td>0.06 (1)</td><td>7.81</td><td></td><td></td></tr><tr><td>L- K</td><td>-21 / 0</td><td>-18.5</td><td>-18.5</td><td>0.02 (4)</td><td>6.25</td><td></td><td></td></tr><tr><td>K- J</td><td>-24 / 0</td><td>-18.5</td><td>-18.5</td><td>0.02 (4)</td><td>6.25</td><td></td><td></td></tr><tr><td>J- I</td><td>-24 / 0</td><td>-18.5</td><td>-18.5</td><td>0.02 (4)</td><td>6.25</td><td></td><td></td></tr><tr><td>I- H</td><td>-21 / 0</td><td>-18.5</td><td>-18.5</td><td>0.02 (4)</td><td>6.25</td><td></td><td></td></tr></table>					CHORDS				WEBS				MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CS1 (LC)	FR-TO		FROM	TO		FR-TO			A- B	0 / 52	-78.0	-78.0	0.15 (1)	10.00	J- D	-237 / 0 0.24 (1)	B- C	0 / 16	-78.0	-78.0	0.09 (1)	10.00	K- C	-132 / 0 0.05 (1)	C- D	0 / 43	-78.0	-78.0	0.05 (1)	10.00	I- E	-132 / 0 0.05 (1)	D- E	0 / 43	-78.0	-78.0	0.05 (1)	10.00			E- F	0 / 16	-78.0	-78.0	0.09 (1)	10.00			F- G	0 / 52	-78.0	-78.0	0.15 (1)	10.00			L- B	-204 / 0	0.0	0.0	0.06 (1)	7.81			H- F	-204 / 0	0.0	0.0	0.06 (1)	7.81			L- K	-21 / 0	-18.5	-18.5	0.02 (4)	6.25			K- J	-24 / 0	-18.5	-18.5	0.02 (4)	6.25			J- I	-24 / 0	-18.5	-18.5	0.02 (4)	6.25			I- H	-21 / 0	-18.5	-18.5	0.02 (4)	6.25			DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 21.0 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.4 PSF TOTAL LOAD = 34.4 PSF SPACING = 24.0 IN. C/C THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018 , ABC 2019 - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014 DESIGN ASSUMPTIONS -OVERHANG NOT TO BE ALTERED OR CUT OFF. (55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD CSI: TC=0.15/1.00 (F-G:1) , BC=0.02/1.00 (I-J:4) , WB=0.24/1.00 (D-J:1) , SSI=0.06/1.00 (F-G:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10 COMPANION LIVE LOAD FACTOR = 1.00 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT . NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.16 (F) (INPUT = 0.90) JSI METAL= 0.09 (B) (INPUT = 1.00)				
CHORDS				WEBS																																																																																																																																		
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CS1 (LC)																																																																																																																															
FR-TO		FROM	TO		FR-TO																																																																																																																																	
A- B	0 / 52	-78.0	-78.0	0.15 (1)	10.00	J- D	-237 / 0 0.24 (1)																																																																																																																															
B- C	0 / 16	-78.0	-78.0	0.09 (1)	10.00	K- C	-132 / 0 0.05 (1)																																																																																																																															
C- D	0 / 43	-78.0	-78.0	0.05 (1)	10.00	I- E	-132 / 0 0.05 (1)																																																																																																																															
D- E	0 / 43	-78.0	-78.0	0.05 (1)	10.00																																																																																																																																	
E- F	0 / 16	-78.0	-78.0	0.09 (1)	10.00																																																																																																																																	
F- G	0 / 52	-78.0	-78.0	0.15 (1)	10.00																																																																																																																																	
L- B	-204 / 0	0.0	0.0	0.06 (1)	7.81																																																																																																																																	
H- F	-204 / 0	0.0	0.0	0.06 (1)	7.81																																																																																																																																	
L- K	-21 / 0	-18.5	-18.5	0.02 (4)	6.25																																																																																																																																	
K- J	-24 / 0	-18.5	-18.5	0.02 (4)	6.25																																																																																																																																	
J- I	-24 / 0	-18.5	-18.5	0.02 (4)	6.25																																																																																																																																	
I- H	-21 / 0	-18.5	-18.5	0.02 (4)	6.25																																																																																																																																	
LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2																																																																																																																																						



TOTAL WEIGHT = 46 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TTW+p	MT20	4.0	6.0		
C	TP+p	MT20	3.0	4.0		
D	TMVW+p	MT20	4.0	6.0	2.00	2.00
F	BMVK+p	MT20	4.0	6.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER							
<u>BEARINGS</u>							
	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
JT							
H	528	0	528	0	0	5-8	1-8
E	340	0	340	0	0	5-8	1-8

UNFACTORED REACTIONS							
1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	374	243 / 0	0 / 0	0 / 0	0 / 0	131 / 0	0 / 0
E	244	142 / 0	0 / 0	0 / 0	0 / 0	102 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING				
TOTAL LOAD CASES: (4)				
C H O R D S				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
FR-TO		FROM	TO	FR-TO
A-B	0 / 52	-78.0	-78.0 0.15 (1)	10.00
B-C	-207 / 0	-78.0	-78.0 0.24 (1)	6.25
C-D	-207 / 0	-78.0	-78.0 0.15 (1)	6.25
H-B	-511 / 0	0.0	0.0 0.06 (1)	7.81
F-D	-404 / 0	0.0	0.0 0.08 (1)	7.81
H-G	0 / 0	-18.5	-18.5 0.13 (4)	10.00
G-F	0 / 0	-18.5	-18.5 0.38 (1)	10.00
F-E	0 / 0	-18.5	-18.5 0.38 (1)	10.00

W E B S				
MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)
G-C	0 / 76	0.03 (4)	B-G	0 / 135
G-D	0 / 165	0.04 (1)		

DESIGN CRITERIA			
SPECIFIED LOADS:			
TOP CH.	LL	=	21.0 PSF
	DL	=	6.0 PSF
BOT CH.	LL	=	0.0 PSF
	DL	=	7.4 PSF
TOTAL LOAD	=	34.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.28")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.28")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.24/1.00 (B-C:1) , BC=0.38/1.00 (E-F:1) , WB=0.04/1.00 (D-G:1) , SSI=0.27/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES				
PLATE	GRIP(DRY)	SHEAR	SECTION	
	(PSI)	(PLI)	(PLI)	
	MAX	MIN	MAX	MIN
MT20	650	371	1747	788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.28 (B) (INPUT = 0.90)
JSI METAL= 0.14 (B) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
346379	H11A	1		JT 45147	E22075349

Alpa Roof Truss, Maple

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ID:9a plwcon izmXpuR1McGqzT9hG-nlAr8dWnPwsWIK?UTUxLDfnKwYKJEUsc5Hs6?pyuVW

Scale = 1:69.7

TOTAL WEIGHT = 256 L

Detailed description: The drawing shows a symmetrical roof truss with a central ridge. The top chord consists of members T1 through T5, and the bottom chord consists of members B1 through B5. Vertical web members are labeled W1 through W10. The truss is supported by a central vertical member T and two side vertical members S and R. The total length of the truss is 39'-10.0". The height of the truss is 7'-0.0". The drawing includes a table of dimensions and a list of members with their respective quantities and ply counts.

MEMBER	QTY	PLY
6x10	1	1
5x8	1	1
5x6	1	1
4x6	1	1
3x2	1	1
2x4	1	1
2x2	1	1
2x1	1	1
2x0	1	1
1x8	1	1
1x6	1	1
1x4	1	1
1x2	1	1
1x1	1	1
1x0	1	1

LUMBER

N. L. G. A. RULES

CHORDS

SIZE

LUMBER

DESCR.

A - C

2x4

DRY

No.2

SPF

C - F

2x6

DRY

No.2

SPF

F - K

2x6

DRY

No.2

SPF

K - M

2x6

DRY

No.2

SPF

M - O

2x4

DRY

No.2

SPF

AA- B

2x6

DRY

No.2

SPF

P - N

2x6

DRY

No.2

SPF

AA- X

2x6

DRY

No.2

SPF

X - T

2x6

DRY

No.2

SPF

T - P

2x6

DRY

No.2

SPF

ALL WEBS

2x3

DRY

No.2

SPF

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT

TYPE

PLATES

W

LEN

Y

X

B

TMVW-t

MT20

5.0

8.0

1.50

2.00

C

TTWW-I

MT20

8.0

9.0

2.75

7.00

D

TMWW+t

MT20

5.0

6.0

2.00

1.50

E

TMWW-t

MT20

5.0

6.0

2.25

2.75

F

TS-t

MT20

4.0

6.0

G

TMW+w

MT20

2.0

4.0

H

TMWW+t

MT20

4.0

6.0

I

TMW+w

MT20

2.0

4.0

J

TMWW-t

MT20

5.0

6.0

2.25

2.75

K

TS-t

MT20

4.0

6.0

L

TMWW+t

MT20

5.0

6.0

2.00

1.50

M

TTWW-I

MT20

8.0

9.0

2.75

7.00

N

TMVW-t

MT20

5.0

8.0

1.50

2.00

P

BMV1+t

MT20

6.0

10.0

Edge

0.50

Q

BMVW-t

MT20

5.0

6.0

2.50

2.75

R

BMWW+t

MT20

5.0

8.0

3.25

1.50

S

BMWW+t

MT20

5.0

8.0

4.00

1.50

T

BS-t

MT20

6.0

7.0

U

BMWWW-t

MT20

8.0

9.0

V

BMWWW-t

MT20

8.0

9.0

W

BMWW+t

MT20

5.0

8.0

4.00

1.50

X

BS-t

MT20

4.0

6.0

Y

BMWW+t

MT20

5.0

8.0

3.25

1.50

Z

BMWW-t

MT20

5.0

6.0

2.50

2.75

AA

BMV1+t

MT20

6.0

10.0

5.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED

GROSS REACTION

DOWN

HORZ

UPLIFT

IN-SX

IN-SX

JT

VERT

HORZ

DOWN

HORZ

UPLIFT

IN-SX

IN-SX

AA

3697

0

3697

0

0

5-8

5-8

P

3208

0

3208

0

0

5-8

4-15

UNFACTORED REACTIONS

1ST LCASE

MAX./MIN. COMPONENT REACTIONS

JT

COMBINED

SNOW

LIVE

PERM.LIVE

WIND

DEAD

SOIL

AA

2633

1621 / 0

0 / 0

0 / 0

0 / 0

1012 / 0

0 / 0

P

2285

1409 / 0

0 / 0

0 / 0

0 / 0

876 / 0

0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) AA, P

BEARING SIZE FACTOR = 1.15 AT JNT(S) AA (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.81 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT D-Y, J-S, L-R

2x3 DRY SPF No.2 T-BRACE AT E-W

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3"

COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MAX. FACTORED

FACTORED

VERT. LOAD LC1

MAX

MAX.

WEBS

MAX. FACTORED

MEMB.

FORCE (LBS)

VERT. (PLF)

UNBRACED LENGTH

FR-TO

MEMB.

FORCE (LBS)

MAX

CSI (LC)

UNBRACED LENGTH

FR-TO

MEMB.

FORCE (LBS)

MAX

CSI (LC)

A-B

0 / 52

-78.0

-78.0

0.17 (1)

10.00

Z-C

-1011 / 0

0.76 (1)

0.93 (1)

B-C

-2877 / 0

-78.0

-78.0

0.33 (1)

3.83

C-Y

0 / 3752

0.96 (1)

0.96 (1)

C-D

-3947 / 0

-78.0

-78.0

0.26 (1)

4.13

Y-D

-2948 / 0

0.96 (1)

0.96 (1)

D-E

-5880 / 0

-78.0

-78.0

0.50 (1)

3.23

D-W

0 / 3241

0.80 (1)

0.80 (1)

E-F

-6723 / 0

-137.2

-137.2

0.73 (1)

2.81

W-E

-1556 / 0

0.94 (1)

0.94 (1)

F-G

-6723 / 0

-137.2

-137.2

0.73 (1)

2.81

E-V

0 / 1212

0.30 (1)

0.30 (1)

G-H

-6723 / 0

-78.0

-78.0

0.46 (1)

3.06

V-G

-529 / 0

0.40 (1)

0.40 (1)

H-I

-6373 / 0

-78.0

-78.0

0.44 (1)

3.16

V-H

0 / 1102

0.27 (1)

0.27 (1)

I-J

-6373 / 0

-78.0

-78.0

0.50 (1)

3.16

H-U

-1032 / 0

0.80 (1)

0.80 (1)

J-K

-4885 / 0

-78.0

-78.0

0.30 (1)

3.74

U-I

-377 / 0

0.28 (1)

0.28 (1)

K-L

-4885 / 0

-78.0

-78.0

0.30 (1)

3.74

U-J

0 / 2139

0.53 (1)

0.53 (1)

L-M

-3352 / 0

-78.0

-78.0

0.21 (1)

4.47

S-J

-1969 / 0

0.64 (1)

0.64 (1)

M-N

-2465 / 0

-78.0

-78.0

0.28 (1)

4.13

S-L

0 / 2572

0.64 (1)

0.64 (1)

N-O

0 / 52

-78.0

-78.0

0.17 (1)

10.00

R-L

-2464 / 0

0.80 (1)

0.80 (1)

AA- B

-3698 / 0

0.0

0.0

0.37 (1)

5.51

R-M

0 / 3164

0.78 (1)

0.78 (1)

P- N

-3206 / 0

0.0

0.0

0.32 (1)

5.87

Q- M

-857 / 0

0.64 (1)

0.64 (1)

B- Z

0 / 2067

0.51 (1)

0.51 (1)

AA- Z

0 / 0

-18.5

-18.5

0.08 (1)

10.00

Q- N

0 / 1771

0.44 (1)

0.44 (1)

Z- Y

0 / 1708

-18.5

-18.5

0.30 (1)

10.00

Y- X

0 / 3947

-18.5

-18.5

0.54 (1)

10.00

X- W

0 / 3947

-18.5

-18.5

0.54 (1)

10.00

W- V

0 / 5880

-33.5

-33.5

0.91 (1)

10.00

V- U

0 / 6530

-18.5

-18.5

0.91 (1)

10.00

U- T

0 / 4885

-18.5

-18.5

0.69 (1)

10.00

T- S

0 / 4885

-18.5

-18.5

0.69 (1)

10.00

S- R

0 / 3352

-18.5

-18.5

0.47 (1)

10.00

R- Q

0 / 1464

-18.5

-18.5

0.26 (1)

10.00

Q- P

0 / 0

-18.5

-18.5

0.07 (1)

10.00

FACTORED CONCENTRATED LOADS (LBS)

JT

LOC.

LC1

MAX-

MAX+

FACE

DIR.

TYPE

HEEL

CONN.

V

19-3-8

-1373

-1373

FRONT

VERT

TOTAL

C1

W

12-11-8

-872

-872

FRONT

VERT

TOTAL

C1

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***

GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.

LOADS WERE DERIVED FROM USER INPUT

NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 21.0 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***

ADD'TL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018 , ABC 2019

- PART 9 OF OBC 2012 (2019 AMENDMENT)

- CSA 086-14

- TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.36")

CALCULATED VERT. DEFL.(LL) = L/ 999 (0.30")

ALLOWABLE DEFL.(TL)= L/360 (1.36")

CALCULATED VERT. DEFL.(TL) = L/ 830 (0.59")

CSI: TC=0.73/1.00 (E-G:1), BC=0.91/1.00 (V-W:1), WB=0.96/1.00 (D-Y:1), SSI=0.30/1.00 (E-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00

COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (Z) (INPUT = 0.90)

JSI METAL= 0.96 (T) (INPUT = 1.00)



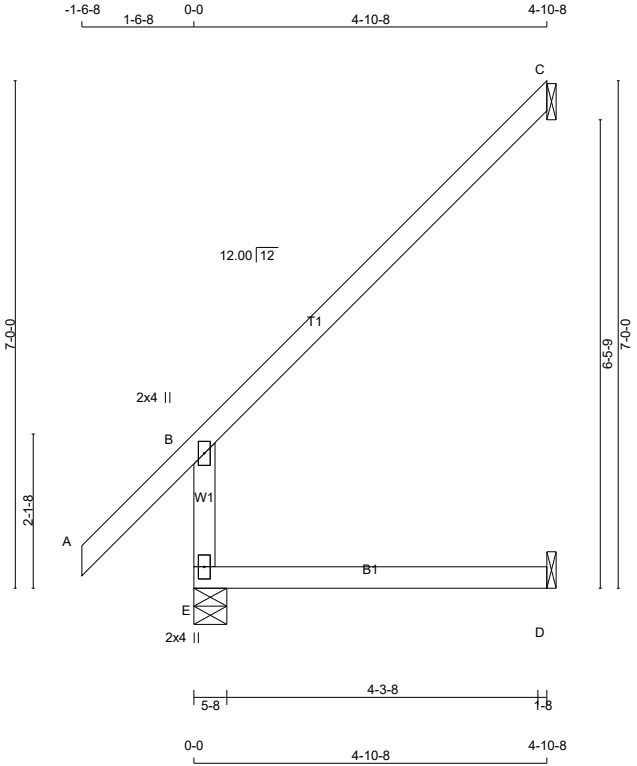
1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
346379	J01		1	TRUSS DESC. JT 45147	E22075350

Alpa Roof Truss, Maple

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ID:9a plwcon izmXpuR1McGqzT9hG-j8HcZJY1xX6EYd9sbuzpl4snwMDUicJvYbLC4iyuWvu



TOTAL WEIGHT = 17 X 18 = 309 lb

LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. E - B 2x4 DRY No.2 SPF A - C 2x4 DRY No.2 SPF E - D 2x4 DRY No.2 SPF DRY: SEASONED LUMBER.										DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS <table><tr><th rowspan="2">JT</th><th colspan="2">FACTORED GROSS REACTION</th><th colspan="2">MAXIMUM FACTORED GROSS REACTION</th><th colspan="2">INPUT BRG</th><th colspan="2">REQRD BRG</th></tr><tr><th>VERT</th><th>HORZ</th><th>DOWN</th><th>HORZ</th><th>UPLIFT</th><th>IN-SX</th><th>IN-SX</th></tr><tr><td>E</td><td>416</td><td>0</td><td>416</td><td>0</td><td>0</td><td>5-8</td><td>1-8</td></tr><tr><td>C</td><td>143</td><td>0</td><td>143</td><td>0</td><td>0</td><td>1-8</td><td>1-8</td></tr><tr><td>D</td><td>40</td><td>0</td><td>44</td><td>0</td><td>0</td><td>1-8</td><td>1-8</td></tr></table> SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D UNFACTORED REACTIONS <table><tr><th rowspan="2">JT</th><th rowspan="2">1ST LCASE</th><th colspan="6">MAX./MIN. COMPONENT REACTIONS</th></tr><tr><th>COMBINED</th><th>SNOW</th><th>LIVE</th><th>PERM.LIVE</th><th>WIND</th><th>DEAD</th><th>SOIL</th></tr><tr><td>E</td><td>294</td><td>197 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>97 / 0</td><td>0 / 0</td></tr><tr><td>C</td><td>99</td><td>77 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>22 / 0</td><td>0 / 0</td></tr><tr><td>D</td><td>32</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>32 / 0</td><td>0 / 0</td></tr></table> BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (4) <table><tr><th colspan="4">CHORDS</th><th colspan="6">WEBS</th></tr><tr><th>MEMB.</th><th>MAX. FACTORED FORCE (LBS)</th><th>FACTORED VERT. LOAD (PLF)</th><th>LC1</th><th>MAX</th><th>MAX. UNBRAC LENGTH</th><th>MEMB.</th><th>MAX. FACTORED FORCE (LBS)</th><th>MAX CSI (LC)</th></tr><tr><td>FR-TO</td><td></td><td>FROM</td><td>TO</td><td></td><td></td><td>FR-TO</td><td></td><td></td></tr><tr><td>E- B</td><td>-366 / 0</td><td>0.0</td><td>0.0</td><td>0.06 (4)</td><td>7.81</td><td></td><td></td><td></td></tr><tr><td>A- B</td><td>0 / 45</td><td>-78.0</td><td>-78.0</td><td>0.15 (1)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>B- C</td><td>-34 / 0</td><td>-78.0</td><td>-78.0</td><td>0.32 (1)</td><td>6.25</td><td></td><td></td><td></td></tr><tr><td>E- D</td><td>0 / 0</td><td>-18.5</td><td>-18.5</td><td>0.10 (4)</td><td>10.00</td><td></td><td></td><td></td></tr></table>										JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG		VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	E	416	0	416	0	0	5-8	1-8	C	143	0	143	0	0	1-8	1-8	D	40	0	44	0	0	1-8	1-8	JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS						COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	E	294	197 / 0	0 / 0	0 / 0	0 / 0	0 / 0	97 / 0	0 / 0	C	99	77 / 0	0 / 0	0 / 0	0 / 0	0 / 0	22 / 0	0 / 0	D	32	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	32 / 0	0 / 0	CHORDS				WEBS						MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	FR-TO		FROM	TO			FR-TO			E- B	-366 / 0	0.0	0.0	0.06 (4)	7.81				A- B	0 / 45	-78.0	-78.0	0.15 (1)	10.00				B- C	-34 / 0	-78.0	-78.0	0.32 (1)	6.25				E- D	0 / 0	-18.5	-18.5	0.10 (4)	10.00				DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 21.0 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.4 PSF TOTAL LOAD = 34.4 PSF SPACING = 24.0 IN. C/C THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018 , ABC 2019 - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014 DESIGN ASSUMPTIONS -OVERHANG NOT TO BE ALTERED OR CUT OFF. (55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL)= L/360 (0.19") CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00") ALLOWABLE DEFL.(TL)= L/360 (0.19") CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02") CSI: TC=0.32/1.00 (B-C:1) , BC=0.10/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.13/1.00 (B-C:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10 COMPANION LIVE LOAD FACTOR = 1.00 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT . NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.28 (B) (INPUT = 0.90) JSI METAL= 0.19 (B) (INPUT = 1.00)									
JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG																																																																																																																																																																								
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX																																																																																																																																																																								
E	416	0	416	0	0	5-8	1-8																																																																																																																																																																								
C	143	0	143	0	0	1-8	1-8																																																																																																																																																																								
D	40	0	44	0	0	1-8	1-8																																																																																																																																																																								
JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS																																																																																																																																																																													
		COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL																																																																																																																																																																							
E	294	197 / 0	0 / 0	0 / 0	0 / 0	0 / 0	97 / 0	0 / 0																																																																																																																																																																							
C	99	77 / 0	0 / 0	0 / 0	0 / 0	0 / 0	22 / 0	0 / 0																																																																																																																																																																							
D	32	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	32 / 0	0 / 0																																																																																																																																																																							
CHORDS				WEBS																																																																																																																																																																											
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)																																																																																																																																																																							
FR-TO		FROM	TO			FR-TO																																																																																																																																																																									
E- B	-366 / 0	0.0	0.0	0.06 (4)	7.81																																																																																																																																																																										
A- B	0 / 45	-78.0	-78.0	0.15 (1)	10.00																																																																																																																																																																										
B- C	-34 / 0	-78.0	-78.0	0.32 (1)	6.25																																																																																																																																																																										
E- D	0 / 0	-18.5	-18.5	0.10 (4)	10.00																																																																																																																																																																										

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

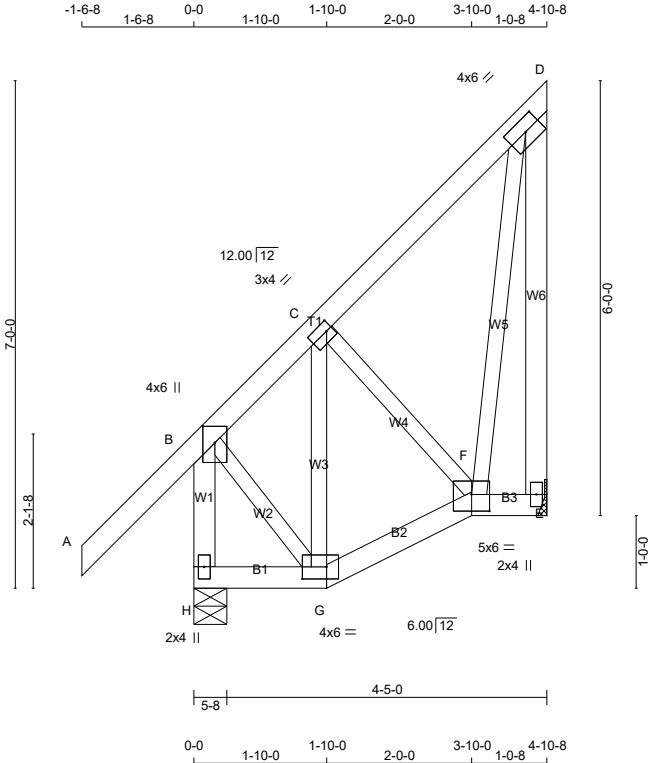


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
346379	J01T		1	TRUSS DESC. JT 45147	E22075351

Alpa Roof Truss, Maple

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ID:9a plwcon izmXpuR1McGqzT9hG-BKr meZgirE49nk39cU2rIP FlapR382nF5mc8yuWyt



TOTAL WEIGHT = 7 X 36 = 250 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
H - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C	TMWW-t	MT20	3.0	4.0	1.50	1.00
D	TMVW-t	MT20	4.0	6.0	2.00	2.75
E	BMV1+p	MT20	2.0	4.0		
F	BBWW-l	MT20	5.0	6.0	2.75	3.00
G	BBWW-l	MT20	4.0	6.0	2.00	4.00
H	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQRD
	GROSS REACTION	GROSS REACTION	DOWN	BRG	BRG
JT	VERT	HORZ	HORZ	UPLIFT	IN-SX
H	385	0	385	0	5-8
E	214	0	214	0	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS		1ST LCASE	MAX./MIN. COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	271	183 / 0	0 / 0	0 / 0	0 / 0	88 / 0	0 / 0
E	153	91 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO	FR-TO
H-B	-368 / 0	0.0 0.0 0.04 (1)	7.81 B-G
A-B	0 / 45	-78.0 -78.0 0.15 (1)	10.00 G-C
B-C	-124 / 0	-78.0 -78.0 0.14 (1)	6.25 C-F
C-D	-43 / 0	-78.0 -78.0 0.10 (1)	6.25 F-D
E-D	-204 / 0	0.0 0.0 0.13 (1)	7.81
H-G	0 / 0	-18.5 -18.5 0.02 (4)	10.00
G-F	0 / 78	-18.5 -18.5 0.03 (4)	10.00
F-E	0 / 0	-18.5 -18.5 0.01 (4)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:			
TOP CH.	LL	=	21.0 PSF
	DL	=	6.0 PSF
BOT CH.	LL	=	0.0 PSF
	DL	=	7.4 PSF
TOTAL LOAD	=	34.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.15/1.00 (A-B:1) , BC=0.03/1.00 (F-G:4) , WB=0.03/1.00 (D-F:1) , SSI=0.08/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

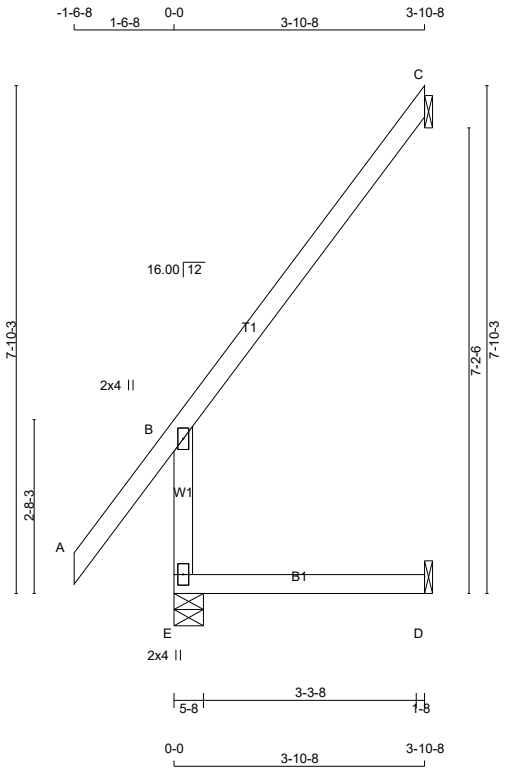
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (B) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2





Scale = 1:35.6

TOTAL WEIGHT = 3 X 18 = 53 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	2.0	4.0	
E	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
E	357	0	357	0	0	5-8	1-8	
C	113	0	113	0	0	1-8	1-8	
D	33	0	37	0	0	1-8	1-8	

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX /MIN. SNOW		LIVE		PERM.LIVE		WIND		DEAD		SOIL	
E	251		171 / 0		0 / 0		0 / 0		0 / 0		80 / 0		0 / 0	
C	79		61 / 0		0 / 0		0 / 0		0 / 0		17 / 0		0 / 0	
D	26		0 / 0		0 / 0		0 / 0		0 / 0		26 / 0		0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

C H O R D S					W E B S				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. (PLF)	LOAD LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM	TO						
E- B	-318 / 0	0.0	0.0	0.03 (4)	7.81				
A- B	0 / 52	-78.0	-78.0	0.18 (5)	10.00				
B- C	-30 / 0	-78.0	-78.0	0.20 (6)	6.25				
E- D	0 / 0	-18.5	-18.5	0.07 (4)	10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL	=	21.0	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.4	PSF
TOTAL LOAD					= 34.4 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.20/1.00 (B-C:6) , BC=0.07/1.00 (D-E:4) ,
WB=0.00/1.00 (n/a:0) , SSI=0.09/1.00 (B-C:6)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES					
PLATE	GRIP(DRY)	SHEAR	SECTION		
	(PSI)	(PLI)	(PLI)		
	MAX	MIN	MAX	MIN	MAX
MT20	650	371	1747	788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (B) (INPUT = 0.90)
JSI METAL= 0.15 (B) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
346379	P05A	1		JT 45147	E22075355

Alpa Roof Truss, Maple

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Mon Jul 25 13:29:48 2022 Page 1
 ID:9a plwcon izmXpuR1McGqzT9hG-0UCF1idQlh?EtiBCVtbT5ZewwAbKrinx9AY4qoyuW

0-0 1-6-0 1-6-0 6-4-7 7-10-7 6-3-3 14-1-10 6-3-3 20-4-13 6-4-7 26-9-5 1-6-0 28-3-4

Scale = 1:47.

6x7 // 16.00 12 20-0 1-10-7 6-1

3x4 = P 2x4 // O 4x6 = N 5x6 = M 3x8 = L 4x6 = K 3x4 = 2x4 //

27-3-13

0-0 1-6-0 1-6-0 6-4-7 7-10-7 6-3-3 14-1-10 6-3-3 20-4-13 6-4-7 26-9-5 1-6-0 28-3-4

TOTAL WEIGHT = 2 X 91 = 182 LB

NUMBER

N. L. G. A. RULES

CHORDS SIZE

A - C 2x4 DRY

C - F 2x4 DRY

F - H 2x4 DRY

H - J 2x4 DRY

B - M 2x4 DRY

M - I 2x4 DRY

ALL WEBS 2x3 DRY

DRY: SEASONED LUMBER.

LUMBER

No.2

No.2

No.2

No.2

No.2

No.2

No.2

DESCR.

SPF

SPF

SPF

SPF

SPF

SPF

SPF

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.75
C	TTWW-h	MT20	6.0	7.0	1.75	4.00
D	TMWW-t	MT20	4.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMWW-t	MT20	4.0	6.0		
H	TTWW-h	MT20	6.0	7.0	1.75	4.00
I	TMB1-I	MT20	3.0	4.0	1.50	2.75
K	BMW1+w	MT20	2.0	4.0		
L	BMWW1-t	MT20	4.0	6.0		
M	BS-t	MT20	3.0	8.0		
N	BMWWW1-t	MT20	5.0	6.0		
O	BMWW1-t	MT20	4.0	6.0		
P	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS

FACTORED REACTION

MAXIMUM FACTORED GROSS REACTION

INPUT BRG

REQRD BRG

BEARING DESIGNER

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
B	93	0	93	0	-11	27-3-13 (11-4-59)	
P	283	0	283	0	0	27-3-13 (11-4-59)	
O	681	0	681	0	0	27-3-13 (11-4-59)	
N	577	0	577	0	0	27-3-13 (11-4-59)	
L	681	0	681	0	0	27-3-13 (11-4-59)	
K	283	0	283	0	0	27-3-13 (11-4-59)	
I	93	0	93	0	-11	27-3-13 (11-4-59)	

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 150 LBS. FACTORED UPLIFT

PROVIDE ANCHORAGE AT BEARING JOINT I FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	61	69 / 0	0 / 0	0 / 0	0 / 0	0 / -8	0 / 0
P	208	93 / 0	0 / 0	0 / 0	0 / 0	115 / 0	0 / 0
O	484	303 / 0	0 / 0	0 / 0	0 / 0	181 / 0	0 / 0
N	412	249 / 0	0 / 0	0 / 0	0 / 0	163 / 0	0 / 0
L	484	303 / 0	0 / 0	0 / 0	0 / 0	181 / 0	0 / 0
K	208	93 / 0	0 / 0	0 / 0	0 / 0	115 / 0	0 / 0
I	61	69 / 0	0 / 0	0 / 0	0 / 0	0 / -8	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, P, O, N, L, K, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A- B	0 / 10	-78.0 -78.0	0.01 (1)	10.00	P- C	-172 / 0	0.03 (1)
B- C	-72 / 0	-78.0 -78.0	0.01 (1)	6.25	C- O	-16 / 0	0.01 (1)
C- D	-12 / 0	-78.0 -78.0	0.46 (1)	6.25	O- D	-558 / 0	0.08 (1)
D- E	0 / 12	-78.0 -78.0	0.46 (1)	10.00	D- N	-25 / 0	0.02 (1)
E- F	0 / 12	-78.0 -78.0	0.46 (1)	10.00	N- E	-450 / 0	0.07 (1)
F- G	0 / 12	-78.0 -78.0	0.46 (1)	10.00	N- G	-25 / 0	0.02 (1)
G- H	-12 / 0	-78.0 -78.0	0.46 (1)	6.25	L- G	-558 / 0	0.08 (1)
H- I	-72 / 0	-78.0 -78.0	0.01 (1)	6.25	L- H	-16 / 0	0.01 (1)
I- J	0 / 10	-78.0 -78.0	0.01 (1)	10.00	K- H	-172 / 0	0.03 (1)
B- P	0 / 40	-18.5 -18.5	0.12 (4)	10.00			
P- O	0 / 28	-18.5 -18.5	0.15 (4)	10.00			
O- N	0 / 12	-18.5 -18.5	0.15 (4)	10.00			
N- M	0 / 12	-18.5 -18.5	0.15 (4)	10.00			
M- L	0 / 12	-18.5 -18.5	0.15 (4)	10.00			
L- K	0 / 28	-18.5 -18.5	0.15 (4)	10.00			
K- I	0 / 40	-18.5 -18.5	0.12 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 21.0 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019

- PART 9 OF OBC 2012 (2019 AMENDMENT)

- CSA 086-14

- TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.46/1.00 (D-E:1) , BC=0.15/1.00 (K-L:4) , WB=0.08/1.00 (D-O:1) , SSI=0.23/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.61 (C) (INPUT = 0.90)

JSI METAL= 0.28 (F) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

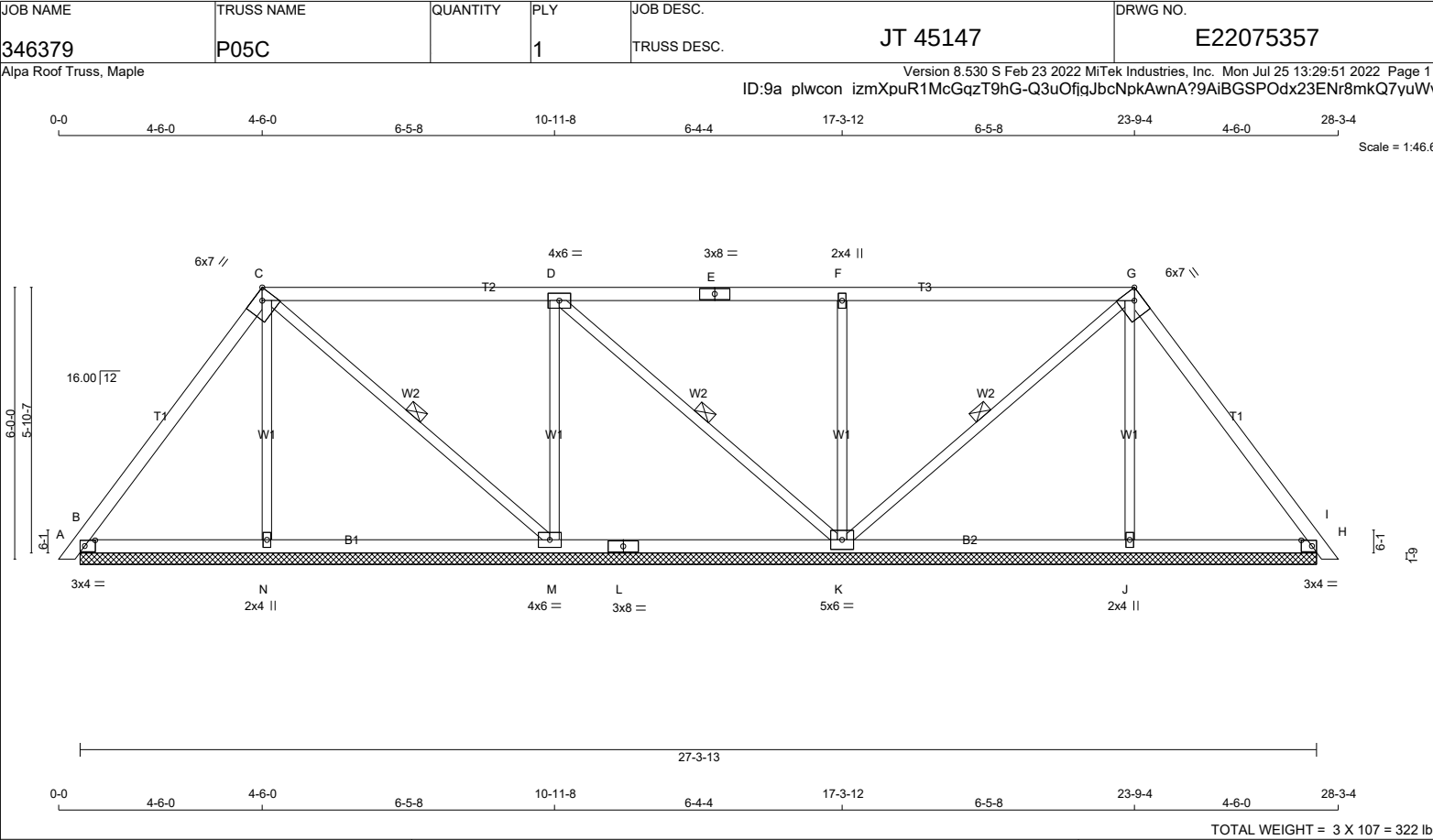
LICENSED PROFESSIONAL ENGINEER

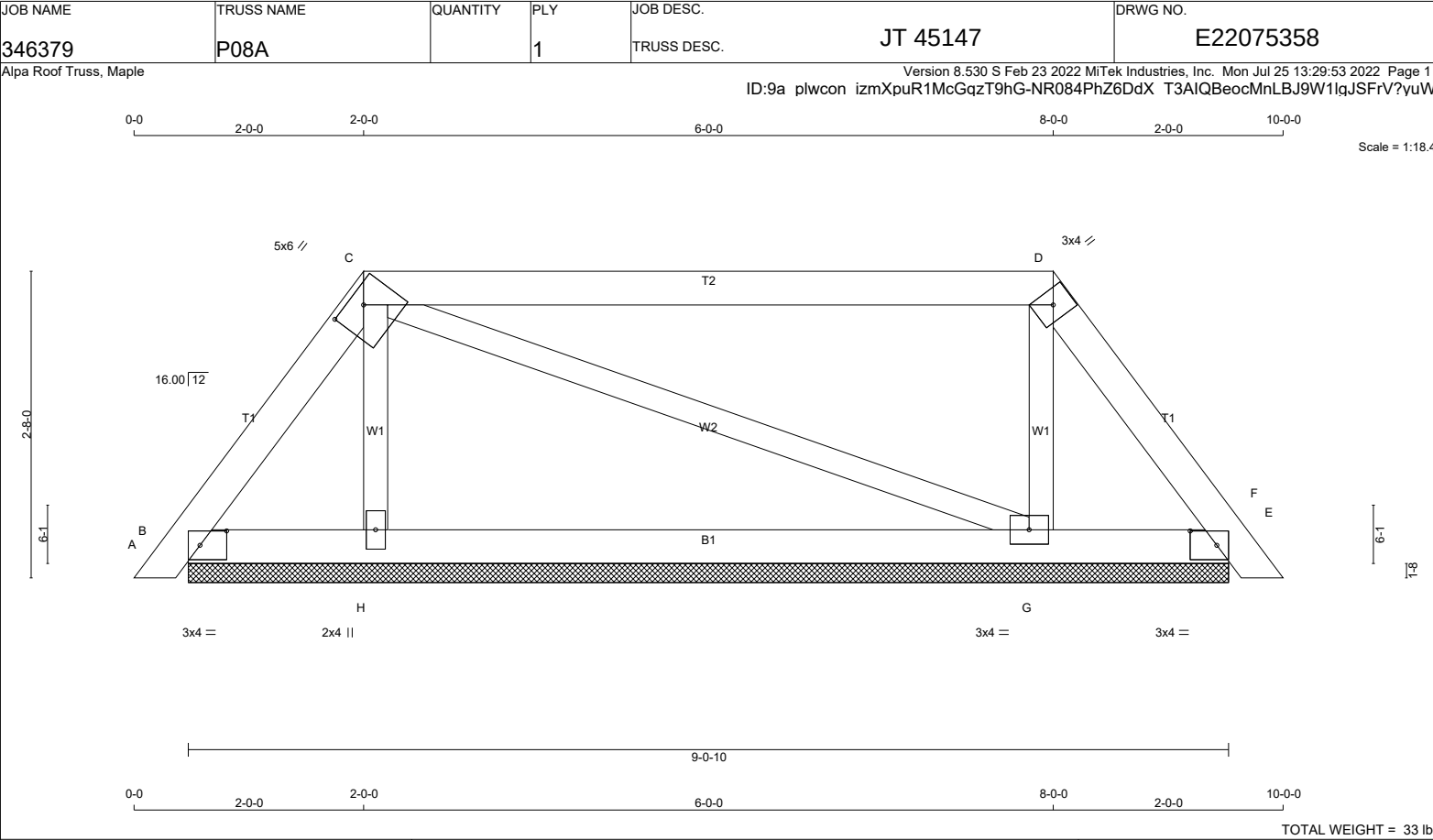
V. WIDYA

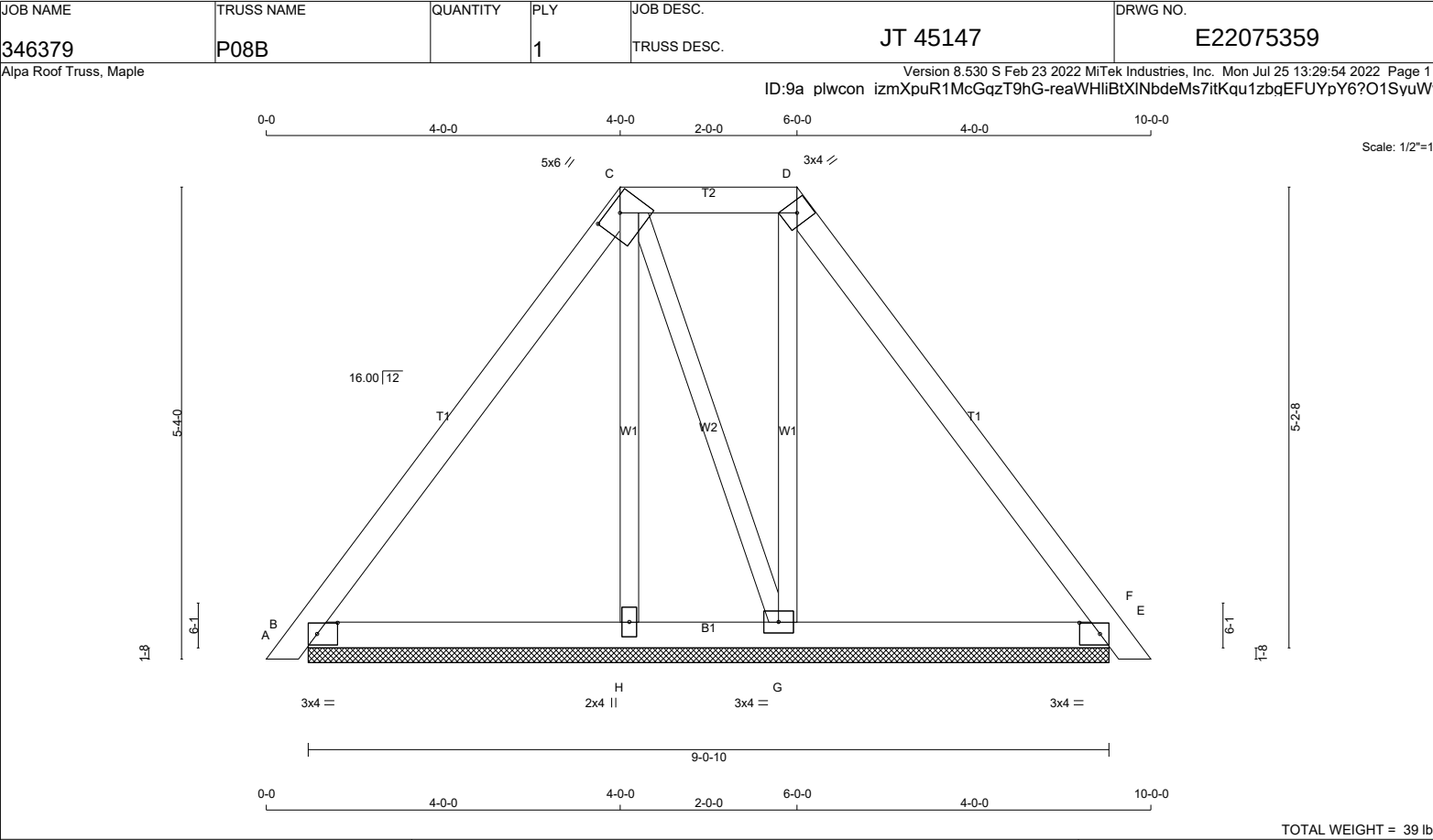
100225448

07/25/2022

PROVINCE OF ONTARIO







TOTAL WEIGHT = 2 X 306 = 612 lb

DRY: SEASONED LUMBER.

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

Y-X	0 / 997	-18.5	-18.5	0.17 (4)	10.00
X-W	0 / 1114	-18.5	-18.5	0.19 (4)	10.00
W-V	0 / 1114	-18.5	-18.5	0.19 (4)	10.00
V-U	0 / 56	-18.5	-18.5	0.10 (4)	10.00
U-T	0 / 59	0.0	0.0	0.08 (1)	10.00
T-F	-519 / 0	0.0	0.0	0.15 (1)	6.25
T-S	0 / 2261	-18.5	-18.5	0.43 (1)	10.00
R-S	0 / 42	0.0	0.0	0.06 (1)	10.00
S-H	-635 / 0	0.0	0.0	0.17 (1)	6.25
R-Q	0 / 43	-18.5	-18.5	0.05 (4)	10.00
Q-P	0 / 1113	-18.5	-18.5	0.17 (1)	10.00
P-O	0 / 1113	-18.5	-18.5	0.17 (1)	10.00
O-N	0 / 997	-18.5	-18.5	0.16 (1)	10.00

JSI GRIP= 0.90 (Y) (INPUT = 0.90)
JSI METAL= 0.48 (C) (INPUT = 1.00)

LUS — Double-Shear Joist Hangers

SIMPSON
Strong-Tie

All LUS hangers have double-shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

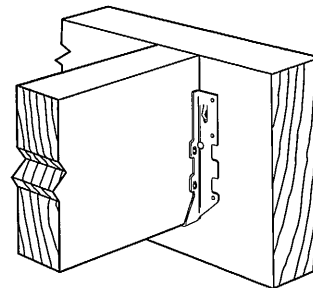
- Factored resistances are in accordance with CSA O86-14 and CSA O86:19.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

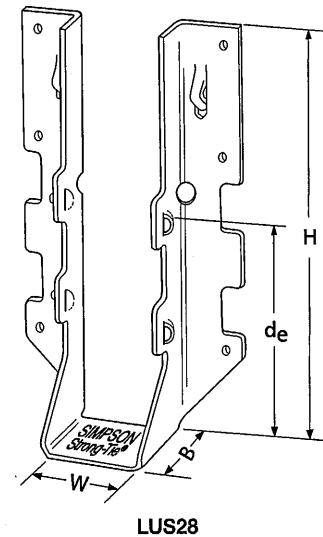
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire,
10d = 0.148" x 3" long common wire
- Double-shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

- These hangers cannot be modified

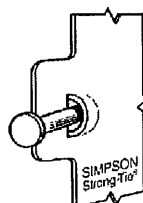


Typical LUS
Installation



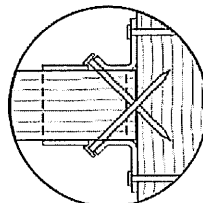
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D-Fir-L		S-P-F	
		W	H	B	d _o ¹	Face	Joist	Uplift (K _o =1.15)	Normal (K _o =1.00)	Uplift (K _o =1.15)	Normal (K _o =1.00)
LUS24	18	1⅞	3⅞	1¼	1⅝	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3⅞	3⅞	2	1⅝	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1⅞	4¾	1¼	3⅝	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3⅞	4¾	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4⅝	4¾	2	3¼	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1⅞	6⅝	1¼	3¾	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3⅞	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4⅝	6¼	2	3¼	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1⅞	7⅞	1¼	3⅞	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3⅞	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4⅝	8⅞	2	5¼	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_o is the distance from the seat of the hanger to the highest joist nail.



Dome double-shear nailing prevents tabs breaking off (available on some models).

US Patent
5,603,580



Double-shear nailing top view.



This technical bulletin is effective until December 31, 2024, and reflects information available as of July 1, 2022. This information is updated periodically and should not be relied upon after December 31, 2024. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECLUS22 7/22 exp. 12/24

(800) 999-5099
strongtie.com

HUS/LJS — Double-Shear Joist Hangers

All hangers have double-shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

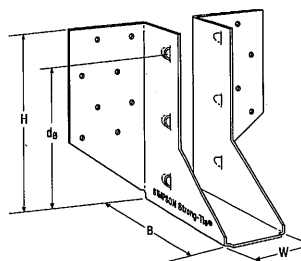
- Factored resistances are in accordance with CSA O86-14 and CSA O86:19.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

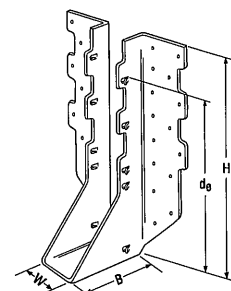
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double-shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

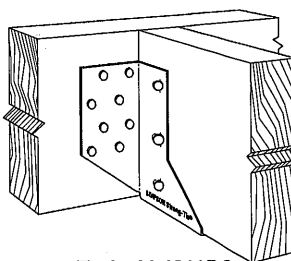
- See current catalogue for options



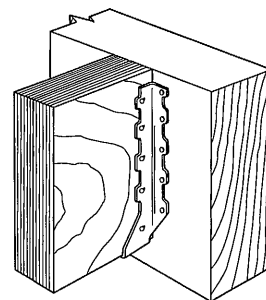
LJS26DS



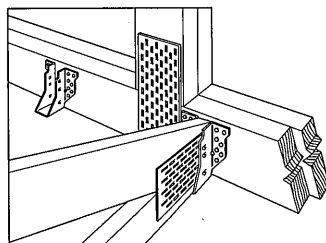
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



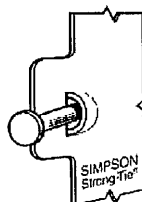
Typical HUS
Installation



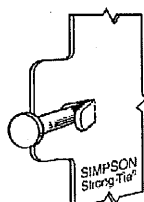
Typical HUS Installation
(Truss designer to provide fastener quantity for connecting multiple members together)

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K ₀ =1.15) lb.	Normal (K ₀ =1.00) lb.	Uplift (K ₀ =1.15) lb.	Normal (K ₀ =1.00) lb.
LJS26DS	18	1⅞	5	3½	4⅝	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1⅝	5⅝	3	3⅞	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1⅝	7⅞	3	6⅞	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1⅝	9⅞	3	7⅞	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1⅞	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

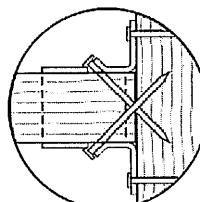
1. d_e is the distance from the seat of the hanger to the highest joist nail.



Dome double-shear nailing prevents tabs breaking off (available on some models).
US Patent 5,603,580



Double-shear nailing side view. Do not bend tab back.



Double-shear nailing top view.



This technical bulletin is effective until December 31, 2024, and reflects information available as of July 1, 2022. This information is updated periodically and should not be relied upon after December 31, 2024. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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HHUS — Double-Shear Joist Hangers

SIMPSON
Strong-Tie

All HHUS hangers have double-shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 14 gauge

Finish: G90 galvanized

Design:

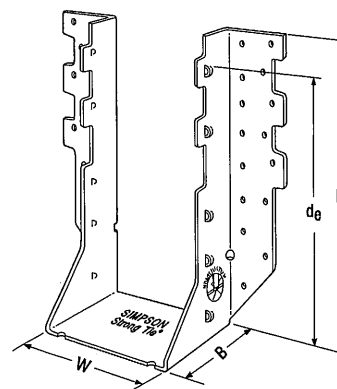
- Factored resistances are in accordance with CSA O86-14 and CSA O86:19.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

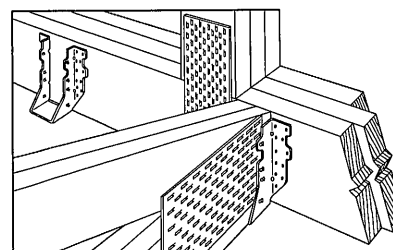
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double-shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

- See current catalogue for options



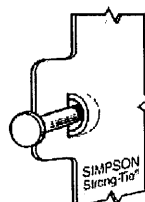
HHUS410



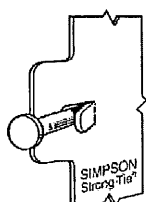
Typical HHUS Installation
(Truss designer to provide fastener quantity for connecting multiple members together)

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _g ¹	Face	Joist	D-Fir-L		S-P-F	
								Uplift (K _g =1.15)	Normal (K _g =1.00)	Uplift (K _g =1.15)	Normal (K _g =1.00)
HHUS26-2	14	3⅞	5⅞	3	3⅞	(14) 16d	(6) 16d	2850	7335	2065	5205
HHUS28-2	14	3⅞	7⅞	3	6⅞	(22) 16d	(8) 16d	3765	8940	2675	6345
HHUS210-2	14	3⅞	9⅞	3	8	(30) 16d	(10) 16d	4670	9660	4235	7000
HHUS210-3	14	4⅞	9	3	7⅞	(30) 16d	(10) 16d	4670	9670	4235	6865
HHUS210-4	14	6⅞	8⅞	3	7⅞	(30) 16d	(10) 16d	4670	10155	4235	7210
HHUS46	14	3⅞	5⅞	3	3⅞	(14) 16d	(6) 16d	2540	7335	2065	5205
HHUS48	14	3⅞	7⅞	3	6⅞	(22) 16d	(8) 16d	3765	8945	2267	6345
HHUS410	14	3⅞	9	3	8	(30) 16d	(10) 16d	4670	9855	4235	7000
HHUS5.50/10	14	5½	9	3	8	(30) 16d	(10) 16d	4670	10155	4235	7210
HHUS7.25/10	14	7¼	9	3⅞	7⅞	(30) 16d	(10) 16d	4670	10155	3370	7210

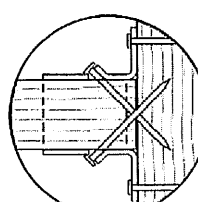
1. d_g is the distance from the seat of the hanger to the highest joist nail.



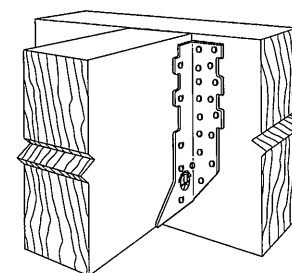
Dome double-shear nailing prevents tabs breaking off (available on some models).
US Patent 5,603,580



Double-shear nailing side view. Do not bend tab back.



Double-shear nailing top view.



Typical HHUS Installation



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TC — Truss Connectors

SIMPSON

Strong-Tie

The TC truss connector is an ideal connector for scissor trusses and can allow horizontal movement up to 1¼". The TC also attaches trusses to top plates or sill plates to resist uplift forces, and is typically used on one or both ends of truss as determined by the building designer.

Material: 16 gauge

Finish: G90 galvanized

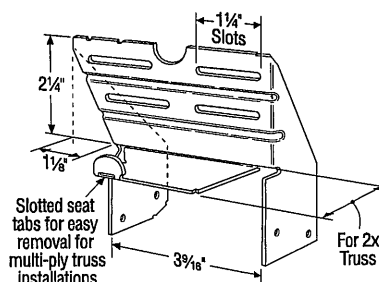
Design: Factored resistances are in accordance with CSA O86-14 and CSA O86:19

Installation:

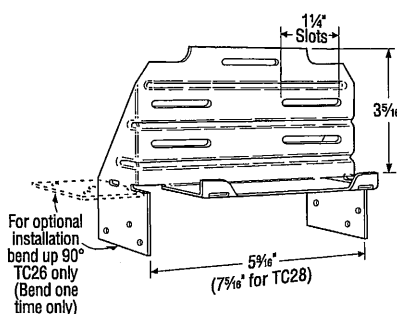
- Use all specified fasteners.
- Nails: 10d = 0.148" dia. x 3" long common wire, 10d x 1½" = 0.148" dia. x 1½" long.
- Drive 10d nails into the truss at the inside end of the slotted holes (inside end is towards the centre of the truss) and clinch on the back side. Do not seat these nails into the truss — allow room under the nail head for movement of the truss with respect to the wall.

Optional TC Installation:

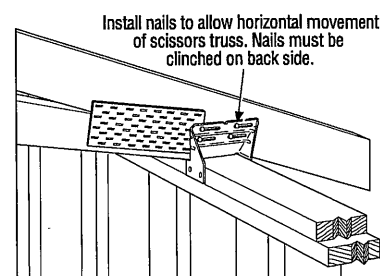
- Bend one flange up 90°. Drive specified nails into the top and face of the top plates or install Titen® screws into the top and face of masonry wall. See optional load tables and installation details.



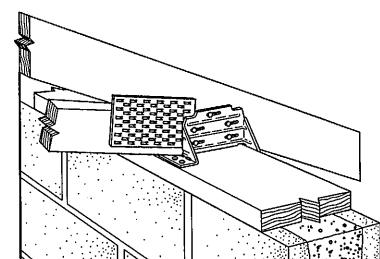
TC24
US Patent 4,932,173



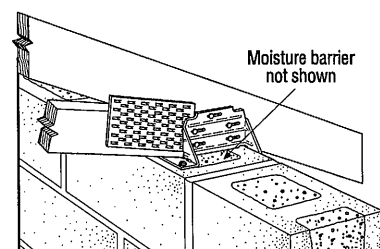
TC26
(TC28 Similar)



Typical TC24 Installation



Optional TC26 Installation for Grouted Concrete Block using a Wood Nailer
(8", 10", 12" Wall Installation Similar)



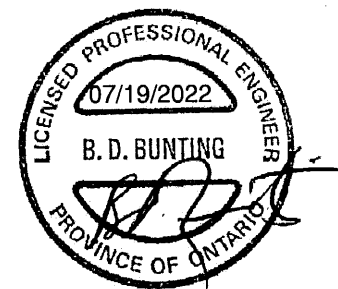
Optional TC26 Installation for Grouted Concrete Block using Titen Screws

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K ₀ =1.15)	Uplift (K ₀ =1.15)
			lb.	lb.
TC24	(4) 10d	(4) 10d	605	430
TC26	(5) 10d	(6) 10d	1015	720
TC28	(5) 10d	(6) 10d	1015	720

Optional TC Installation Table

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K ₀ =1.15)	Uplift (K ₀ =1.15)
			lb.	lb.
TC26	(5) 10d	(6) 10d x 1½"	810	660
	(5) 10d	(6) 10d	930	660

1. Factored resistances have been increased 15% for earthquake or wind loading; no further increase allowed; reduce where other loads govern.
2. Grout strength is 15 MPa minimum.
3. Optional TC26 installation with 10d nails requires minimum 3" top plate thickness.
4. TC26 fastened to grouted concrete block with (6) — ¾" x 2¼" Titen screws has a factored uplift resistance of 275 lb.

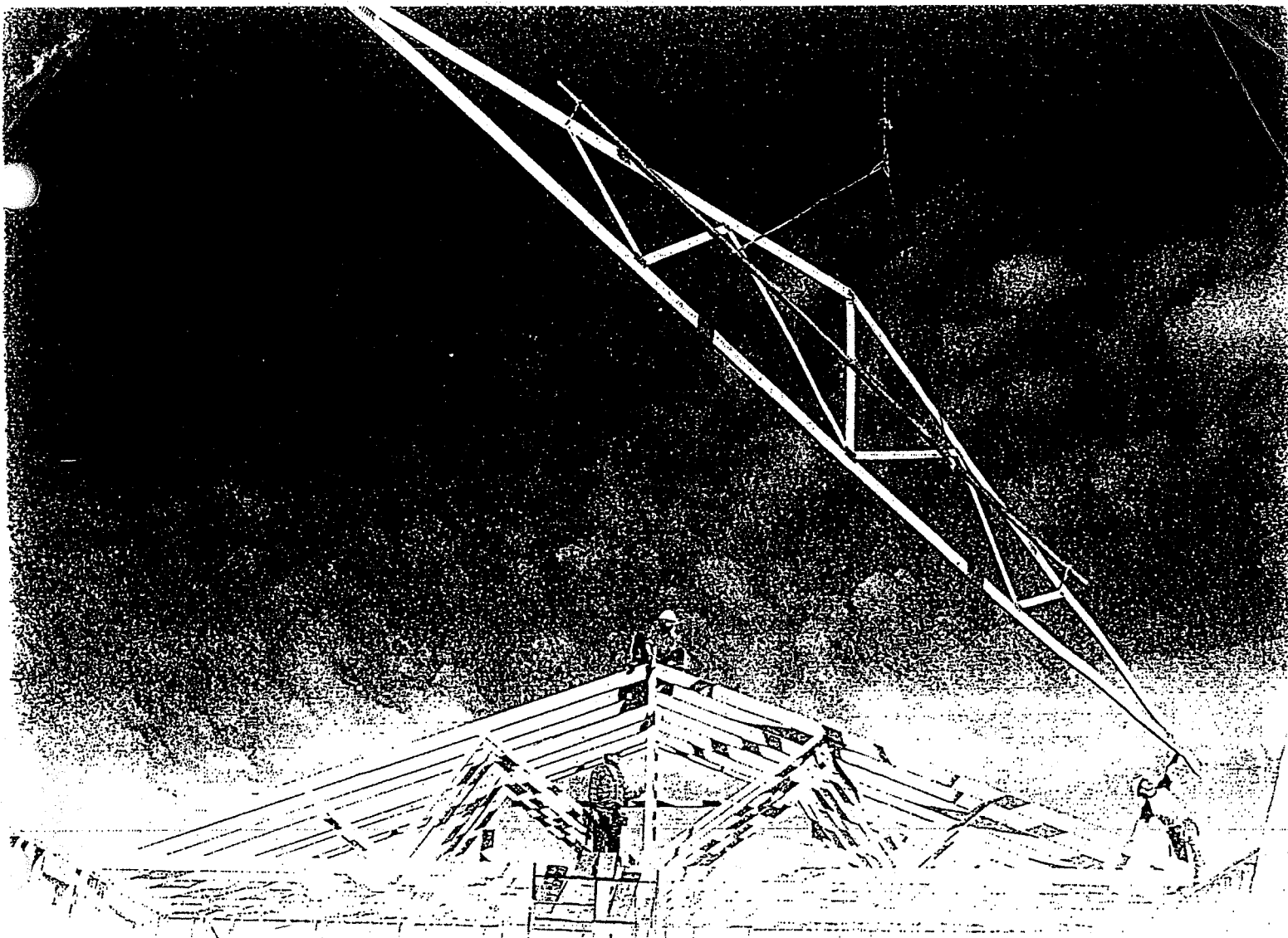


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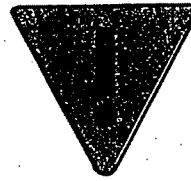
Wood Truss Installation

**A Guide to proper handling, erecting and bracing
metal plate connected wood trusses**

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2 Job Site Handling.....	5
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Warning



General

Familiarity with the Construction Design Documents, the Truss Design Drawings, and Truss Placement Plans (if required by the Construction Design Documents) is required to properly erect, brace, and connect the trusses to the building system.

All of the care and quality involved in the design and manufacture of wood trusses can be jeopardized if the trusses are not properly handled, erected, and braced.

The consequences of improper handling, erecting, and bracing may be a collapse of the structure, which at best is a substantial loss of time and materials, and at worst is a loss of life. The majority of truss accidents occur during truss installation and not as a result of improper design or manufacture.

Prior to truss erection, the builder/erector shall meet with the erection crew for a safety and planning meeting, making sure each crew member understands his or her roles and responsibilities during the erection process.

Temporary Erection Bracing

Trusses are not marked in any way to identify the frequency, or location of temporary erection bracing.

All temporary bracing shall comply with the latest edition of *Commentary and Recommendations for Handling, Installing & Bracing Metal Plate Connected Wood Trusses* (HIB), published by the Truss Plate Institute, and/or as specified in the Construction Design Documents prepared by the building designer.

Permanent Truss Bracing

Permanent bracing for the roof or floor trusses is the responsibility of the building designer and should be shown on the Construction Design Documents. Permanent bracing locations for individual compression members of a wood truss are shown on the Truss Design Drawings, and shall be installed by the building or erection contractor. This bracing is needed for the proper performance of individual trusses within the roof or floor system. The design and connection of the bracing to the truss and then to the overall building system is the responsibility of the building designer, and is in addition to the permanent bracing plan, which is also specified by the building designer.

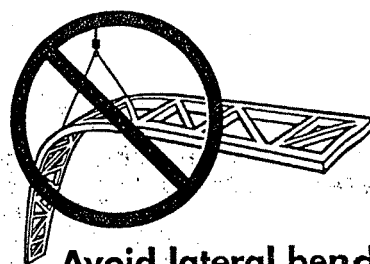
Special Design Requirements

Special design requirements, such as wind bracing, portal bracing, seismic bracing, diaphragms, shear walls, or other load transfer elements and their connections to wood trusses must be considered separately by the building designer, who shall determine size, location, and method of connections for all bracing as needed to resist these forces.

1 Unloading & Lifting

Never handle trusses flat

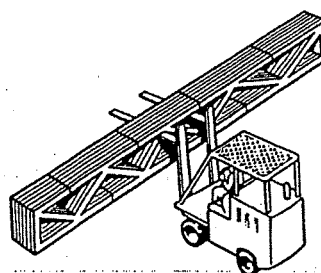
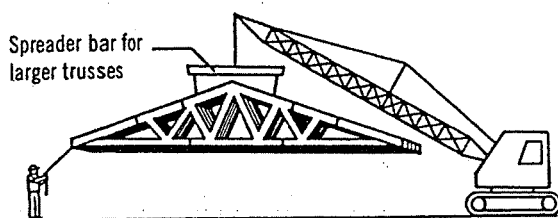
Beginning with the unloading process, and throughout all phases of construction, care must be taken to avoid lateral bending of trusses, which can cause damage to the lumber and metal connector plates at the joints.



Avoid lateral bending

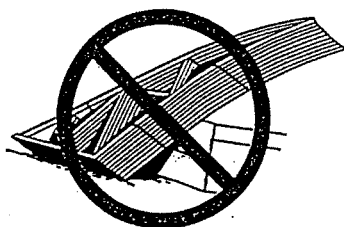
- Use special care in windy weather.
- If using a crane within 10 feet of an electric line, contact the local power company.
- If using a crane within 5 miles of an airport, contact the airport 30 days prior to erection to learn about any safety regulations that must be followed.

2 Job Site Handling



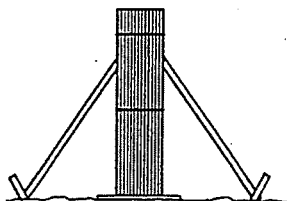
All trusses should be picked up at the top chords in a vertical position only

Proper banding and smooth ground allow for unloading of trusses without damage. This should be done as close to the building site as possible to minimize handling. Do not break banding until installation begins. Hand erection of trusses is allowed, provided excessive lateral bending is prevented.



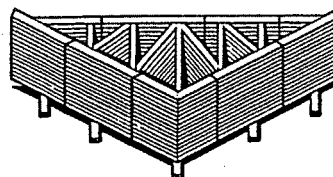
Do not store unbraced bundles upright

If trusses are stored vertically they shall be braced in a manner that will prevent tipping or topping. Generally cutting of the banding is done just prior to installation.



Do not store on uneven ground

If trusses are stored horizontally, blocking should be used on eight to ten foot centers, or as required, to minimize lateral bending and moisture gain.

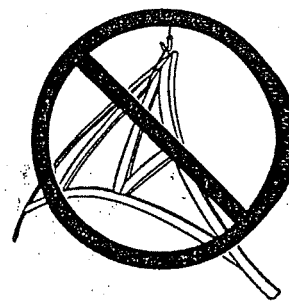


Care should be exercised when removing banding to avoid damaging trusses.

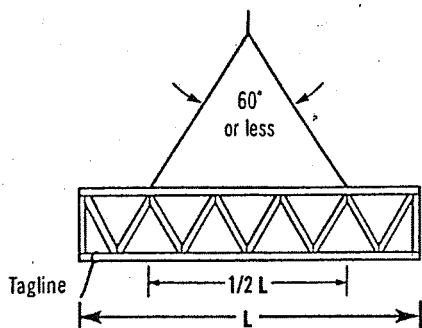
During long term storage, trusses shall be protected from the environment in a manner that provides for adequate ventilation of the trusses. If tarpaulins or other material is used, the ends shall be left open for ventilation. Plastic is not recommended, since it can trap moisture.

3 Hoisting

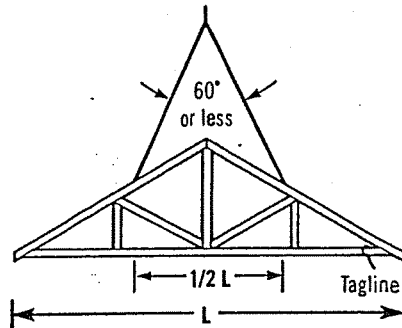
All trusses that are erected one at a time shall be held safely in position by the erection equipment until such time as all necessary bracing has been installed and the ends of the trusses are securely fastened to the building.



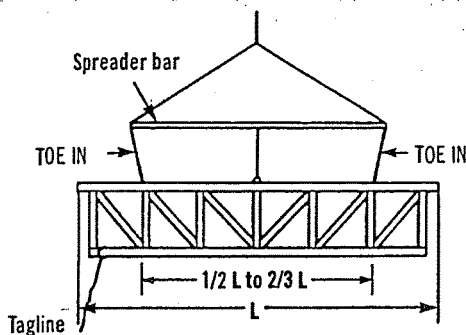
Avoid lateral bending



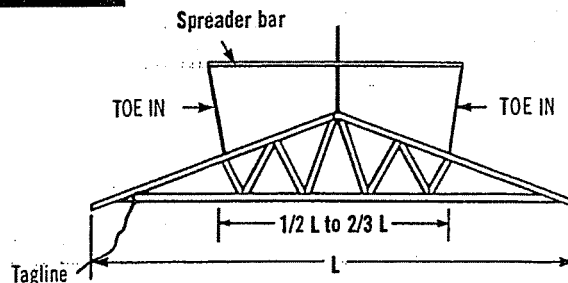
$L \leq 30'$



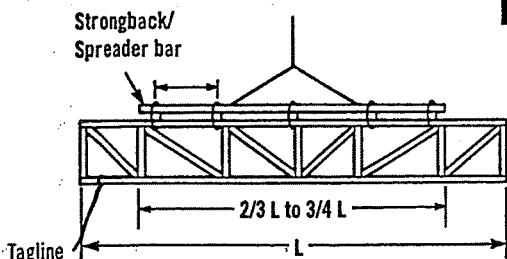
Truss sling is acceptable where these criteria are met.



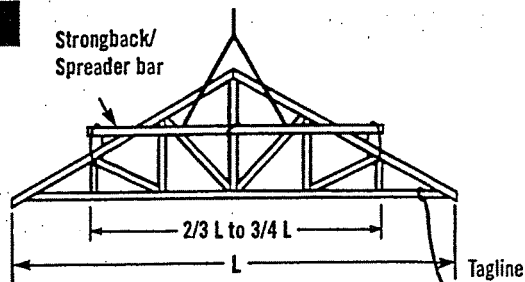
$30' < L \leq 60'$



Use spreader bar in all other cases. It should be noted that the lines from the ends of the spreader bar "TOE IN"; if these lines should "TOE OUT" the truss may fold in half.



$L > 60'$



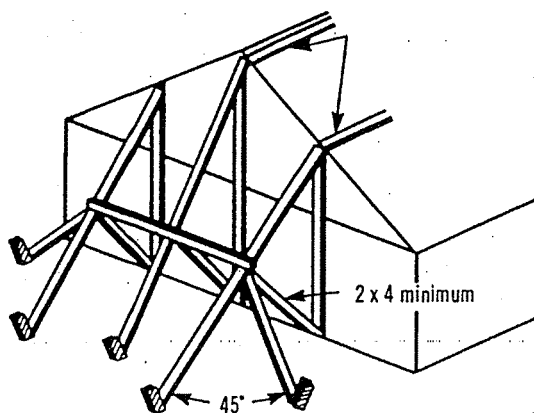
For lifting trusses with spans in excess of 60 feet, it is recommended that a strongback/spreader bar be used as illustrated. The strongback/spreader bar should be attached to the top chord and web members at intervals of approximately 10 feet. Further, the strongback/spreader bar should be at or above the mid-height of the truss to prevent overturning. The strongback/spreader bar can be of any material with sufficient strength to safely carry the weight of the truss and sufficient rigidity to adequately resist bending of the truss.

4 Beginning the Erection Process

It is important for the builder or erection contractor to provide substantial bracing for the first truss erected. The two or more trusses making up the rest of the first set are tied to and rely upon the first truss for stability. Likewise, after this first set of trusses is adequately cross-braced, the remaining trusses installed rely upon this first set for stability. Thus, the performance of the truss bracing system depends to a great extent on how well the first group of trusses is braced.

Ground Brace - Exterior

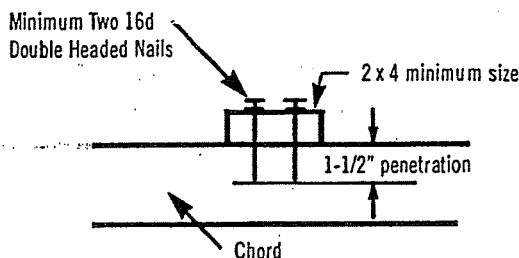
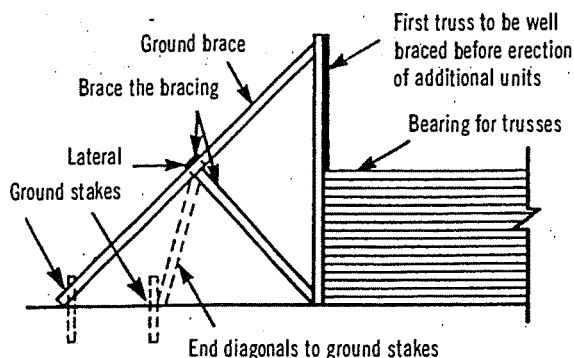
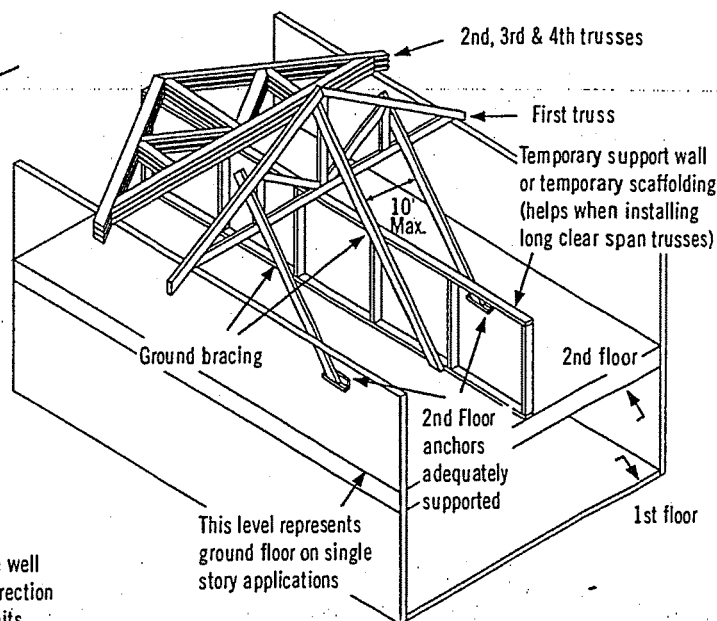
One satisfactory method ties the first unit of trusses off to a series of braces that are attached to a stake driven into the ground and securely anchored. The ground brace itself should be supported as shown below or it is apt to buckle. Additional ground braces in the opposite direction, inside the building, are also recommended.



Note: Locate ground braces for first truss directly in line with all rows of top chord continuous lateral bracing (either temporary or permanent).

Ground Brace - Interior

Another satisfactory method where height of building or ground conditions prohibit bracing from the exterior is to tie the first truss rigidly in place from the interior at the floor level, provided the floor is substantially completed and capable of supporting the ground bracing forces. Securely fasten the first truss to the middle of the building. Brace the bracing similar to exterior ground bracing shown at left. Set trusses from the middle toward the end of the building. Properly cross-brace the first set of trusses before removing floor braces and setting remaining trusses.

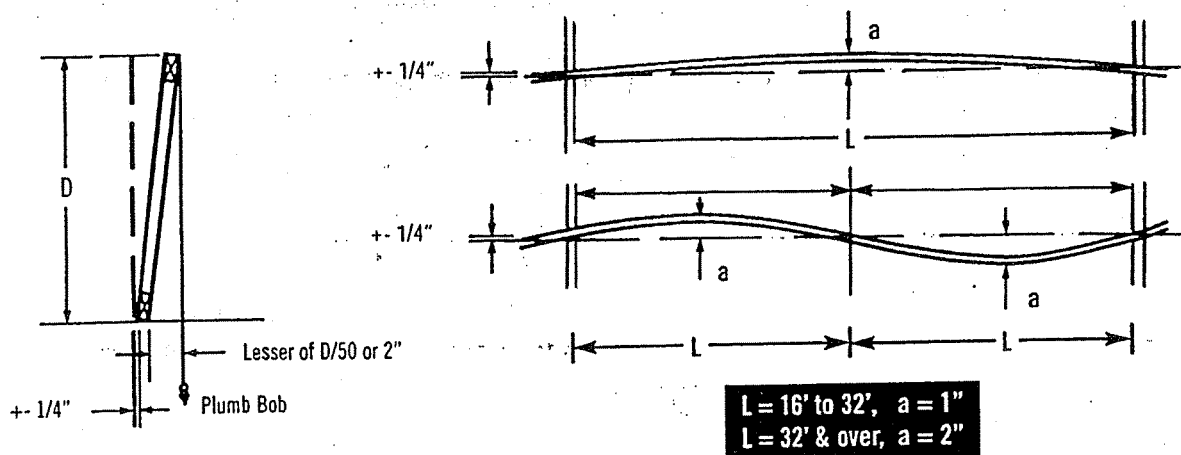


Inadequate size of bracing material or inadequate fastening is a major cause of erection dominoing.

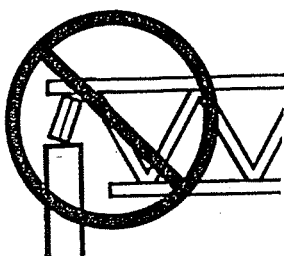
5 Erection Tolerance

Complying with erection tolerances is critical to achieving an acceptable roof or floor line, and to accomplishing effective bracing. Setting trusses within tolerance the first time will prevent the need for the hazardous practice of respacing or adjusting trusses when roof sheathing or roof purlins are installed. Trusses leaning or bowing can cause nails to miss the top chords when sheathing is applied, and create cumulative stresses on the bracing, which is a frequent cause of dominoing.

When sheathing, make sure nails are driven into the top chord of the trusses.



6 Bracing



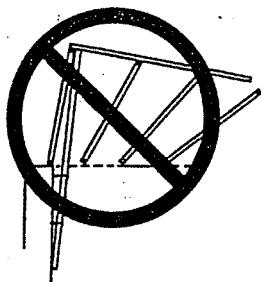
Do not install trusses on temporarily connected supports



Do not walk on unbraced trusses



Do not walk on trusses or gable ends lying flat

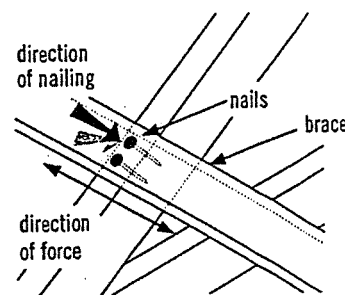


Nails in withdrawal (parallel to force)

All anchors, hangers, tie-downs, seats, bearing ledgers, etc., that are part of the supporting structure shall be accurately and properly placed and permanently attached before truss installation begins. No trusses shall ever be installed on anchors or ties that have temporary connections to the supporting structure.

Nailing scabs to the end of the building to brace the first truss is not recommended.

All nailing of bracing should be done so that nails are driven perpendicular to the direction of force, as shown at right.



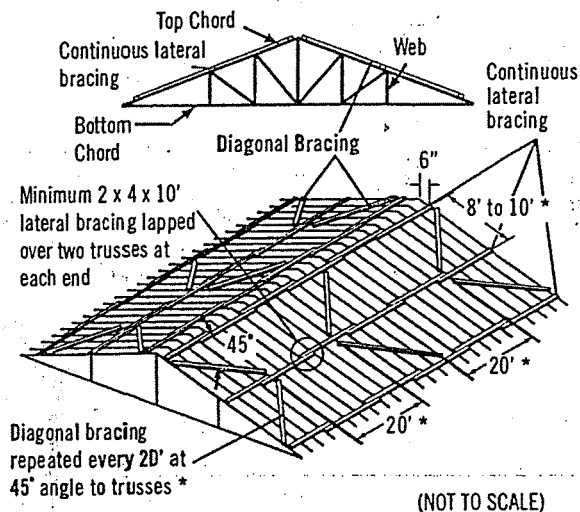
Well nailed (perpendicular to force)

7 Bracing Requirements for 3 Planes of Roof

Temporary erection bracing must be applied to three planes of the roof system to ensure stability: Plane 1) Top Chord (sheathing), Plane 2) Bottom Chord (ceiling plane), and Plane 3) Web Member plane or vertical plane perpendicular to trusses.

1) Top Chord Plane

Most important to the builder or erection contractor is bracing in the plane of the top chord. Truss top chords are susceptible to lateral buckling before they are braced or sheathed.

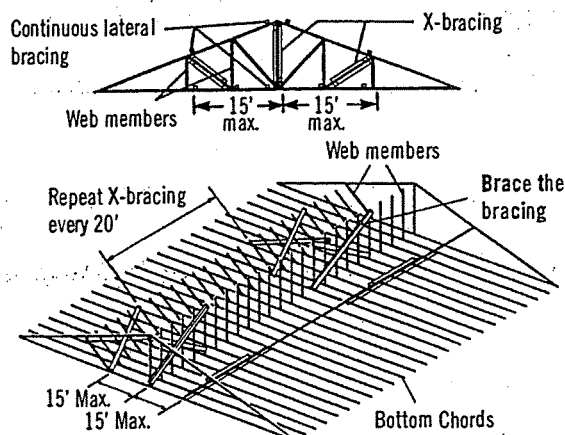


Exact spacing between trusses should be maintained as bracing is installed to avoid the hazardous practice of removing bracing to adjust spacing. This act of "adjusting spacing" can cause trusses to topple if connections are removed at the wrong time.

3) Web Member Plane

"X" bracing, as shown, is critical in preventing trusses from leaning or dominoing. Repeat as shown to create a succession of rigid units.

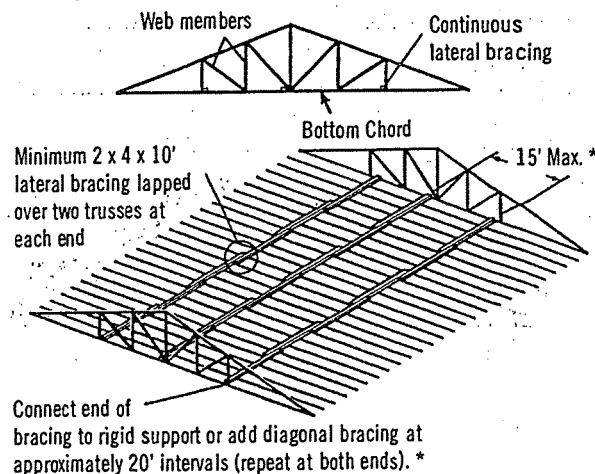
X-bracing should be installed on vertical web members wherever possible, at or near lateral bracing. Plywood or OSB may be substituted for X-bracing.



Note: Top chords and some web members are not shown, in order to make drawings more readable.

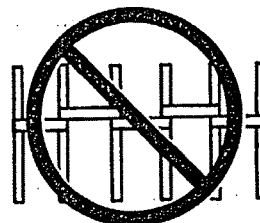
2) Bottom Chord Plane

In order to hold proper spacing on the bottom chord, temporary bracing is recommended on the top of the bottom chord.

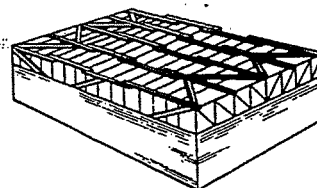


* Long spans, heavy loads or other spacing configurations may require closer spacing between lateral bracing and closer intervals between diagonals. Consult the building designer or HIB and DSB (Recommended Design Specification for Temporary Bracing of Metal-Plate Connected Wood Trusses) for details.

Diagonal or cross-bracing is very important!



Do not use short blocks to brace individual trusses without a specific bracing plan detailing their use

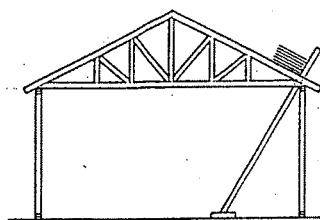
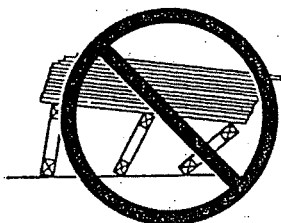


Bracing requirements using the same principles apply to parallel chord trusses

8 Stacking Materials

Do not proceed with building completion until all bracing is securely and properly in place

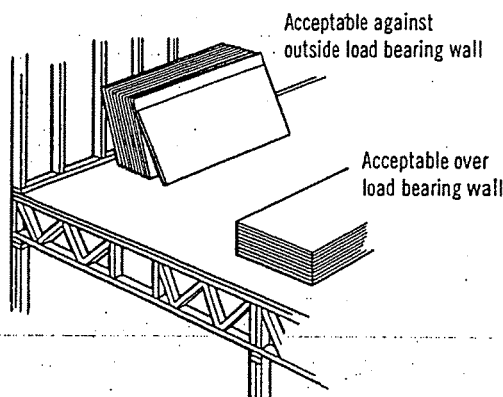
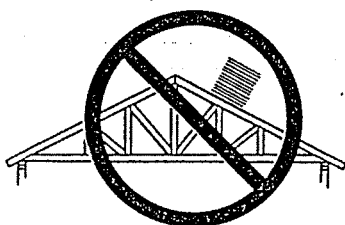
Never stack materials on unbraced or inadequately braced trusses



Platform must be rigidly braced

Proper distribution of construction materials is a must during construction.

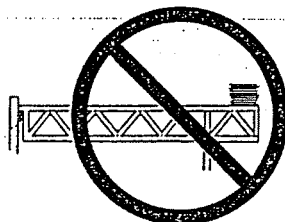
Never stack materials near a peak



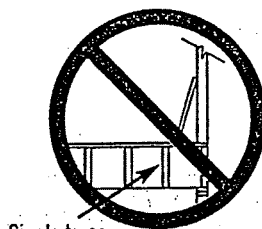
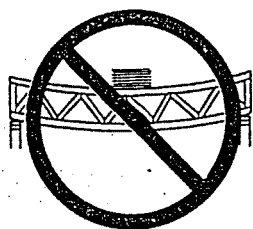
Acceptable against outside load bearing wall

Acceptable over load bearing wall

Never stack materials on the cantilever of a truss

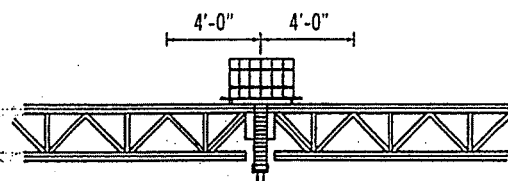


Always stack materials over two or more trusses.



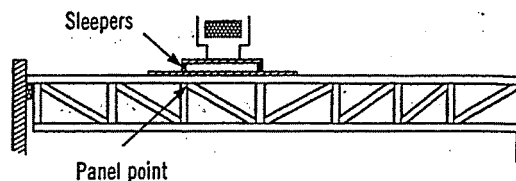
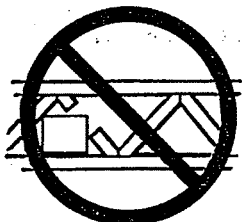
Single truss

Never overload small groups or single trusses. Position load over as many trusses as possible.



Roofing and mechanical contractors are cautioned to stack materials only along outside supporting members or directly over inside supporting members. Trusses are not designed for dynamic loads (i.e., moving vehicles). Extreme care should be taken when loading and stacking construction materials (rolled roofing, mechanical equipment, etc.) on the roof or floor system.

Never cut any structural member of a truss.



Sleepers

Panel point

Sleepers for mechanical equipment should be located at panel points (joints) or over main supporting members, and only on trusses that have been designed for such loads.

Caution Notes

Errors in building lines and/or dimensions, or errors by others shall be corrected by the contractor or responsible construction trade subcontractor or supplier **before** erection of trusses begins.

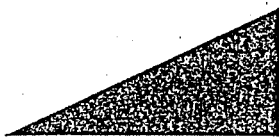
Cutting of nonstructural overhangs is considered a part of normal erection and shall be done by the builder or erection contractor.

Any field modification that involves the cutting, drilling, or relocation of any structural truss member or connector plate shall not be done without the approval of the truss manufacturer or a licensed design professional.

The methods and procedures outlined are intended to ensure that the overall construction techniques employed will put floor and roof trusses safely in place in a completed structure. These recommendations for bracing wood trusses originate from the collective experience of leading technical personnel in the wood truss industry, but must, due to the nature of responsibilities involved, be presented only as a guide for use by a qualified building designer, builder, or erection contractor. Thus, the Wood Truss Council of America expressly disclaims any responsibility for damages arising from the use, application, or reliance on the recommendations and information contained herein.

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