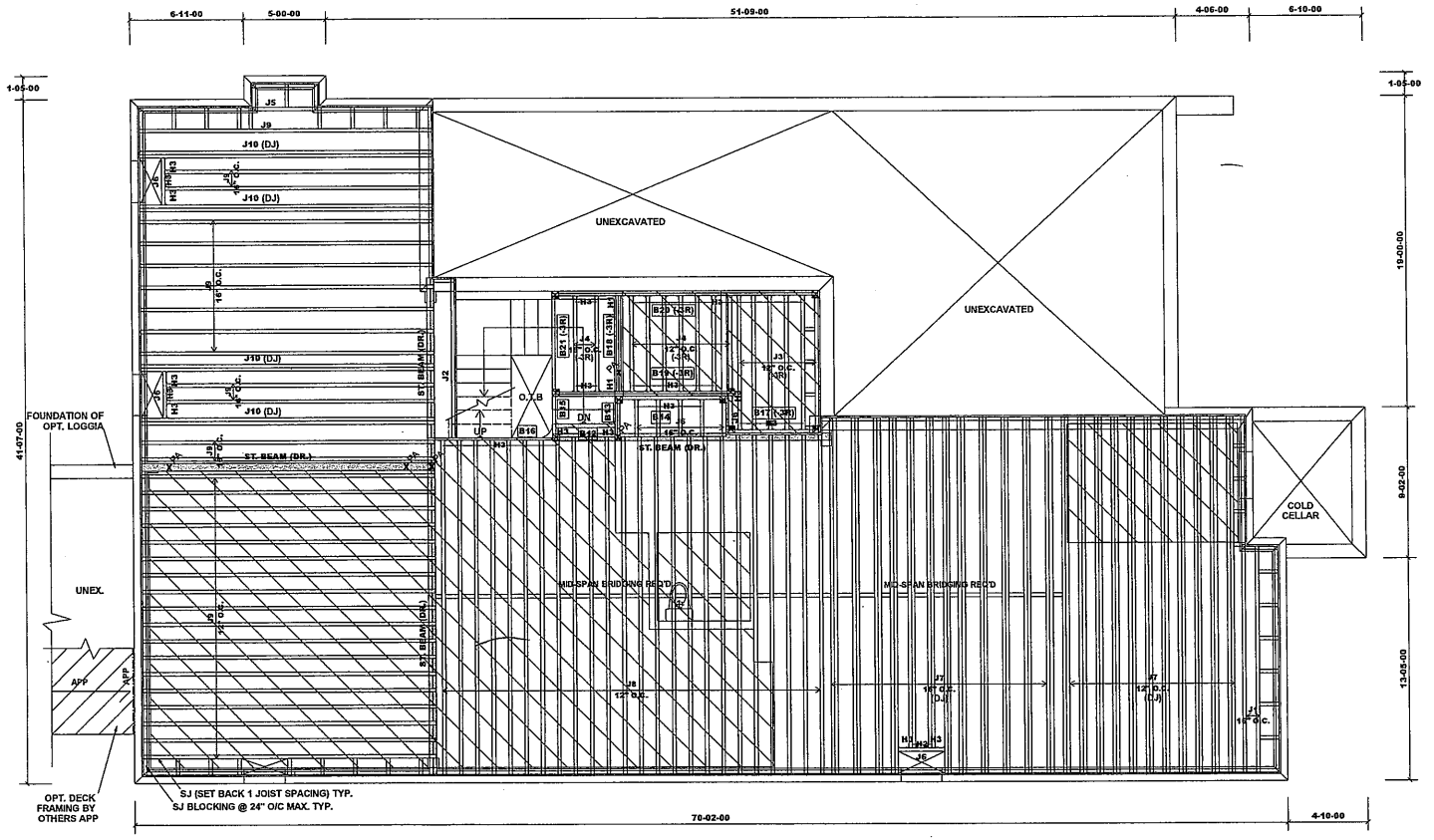


Products				
PlotID	Length	Product	Plies	Net Qty
B12	4-00-00	11 7/8" NI-20	1	1
B13	3-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B14	7-00-00	11 7/8" NI-20	1	1
B15	3-00-00	11 7/8" NI-20	1	1
B16	9-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	1	1
B17 (-3R)	6-00-00	11 7/8" NI-20	1	1
B18 (-3R)	6-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B19 (-3R)	11-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B20 (-3R)	17-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B21 (-3R)	7-00-00	11 7/8" NI-20	1	1
J1	14-00-00	11 7/8" NI-20	1	2
J2	10-00-00	11 7/8" NI-20	1	1
J3	9-00-00	11 7/8" NI-20	1	6
J4	6-00-00	11 7/8" NI-20	1	9
J5	4-00-00	11 7/8" NI-20	1	1
J6	3-00-00	11 7/8" NI-20	1	9
J7	22-00-00	11 7/8" NI-40x	2	46
J8	21-00-00	11 7/8" NI-40x	1	24
J9	18-00-00	11 7/8" NI-40x	1	32
J10	18-00-00	11 7/8" NI-40x	2	8
Ca1	222-00-00	1 1/8" x 11 7/8" Rim Board	1	1
Bk1	103-00-00	11 7/8" NI-20	1	1

Connector Summary			
PlotID	Qty	Manuf	Product
H1	2		HGUS410
H2	2		HU312-2
H3	53		LT251188

GROUND FLOOR FRAMING
UNIT 5013 - THE RIVERVIEW
ELEVATION A
+ W/ W.O.D. CONDITION

FLOOR LOADING
LIVE LOAD : 40 PSF
DEAD LOAD : 15 PSF
DEAD LOAD (TILE): 20 PSF

HATCH LEGEND
Ceramic Tile
Conv Framed

APP - AS PER PLAN
BBO - BEAM BY OTHERS
PA - POST ABOVE
O.T.B - OPEN TO BELOW
GT - GIRDER TRUSS
RT - ROOF TRUSS

RIMBOARD
1-1/8" X 11-7/8" O.S.B

SUBFLOOR: 3/4" NAILED & GLUED*

Blocking panels are required over all interior supports.
Squash blocks are required under concentrated loads.

Ceramic Tile Application as per O.B.C. 9.30.6

Provide I-Joist blocking between cantilevered joists (along bearing) and rimboard closure at ends.

Do not scale - refer to architectural plans for dimensions.

S3046658-S3046701

JT/PL: 45147/116460
LI: 343077*

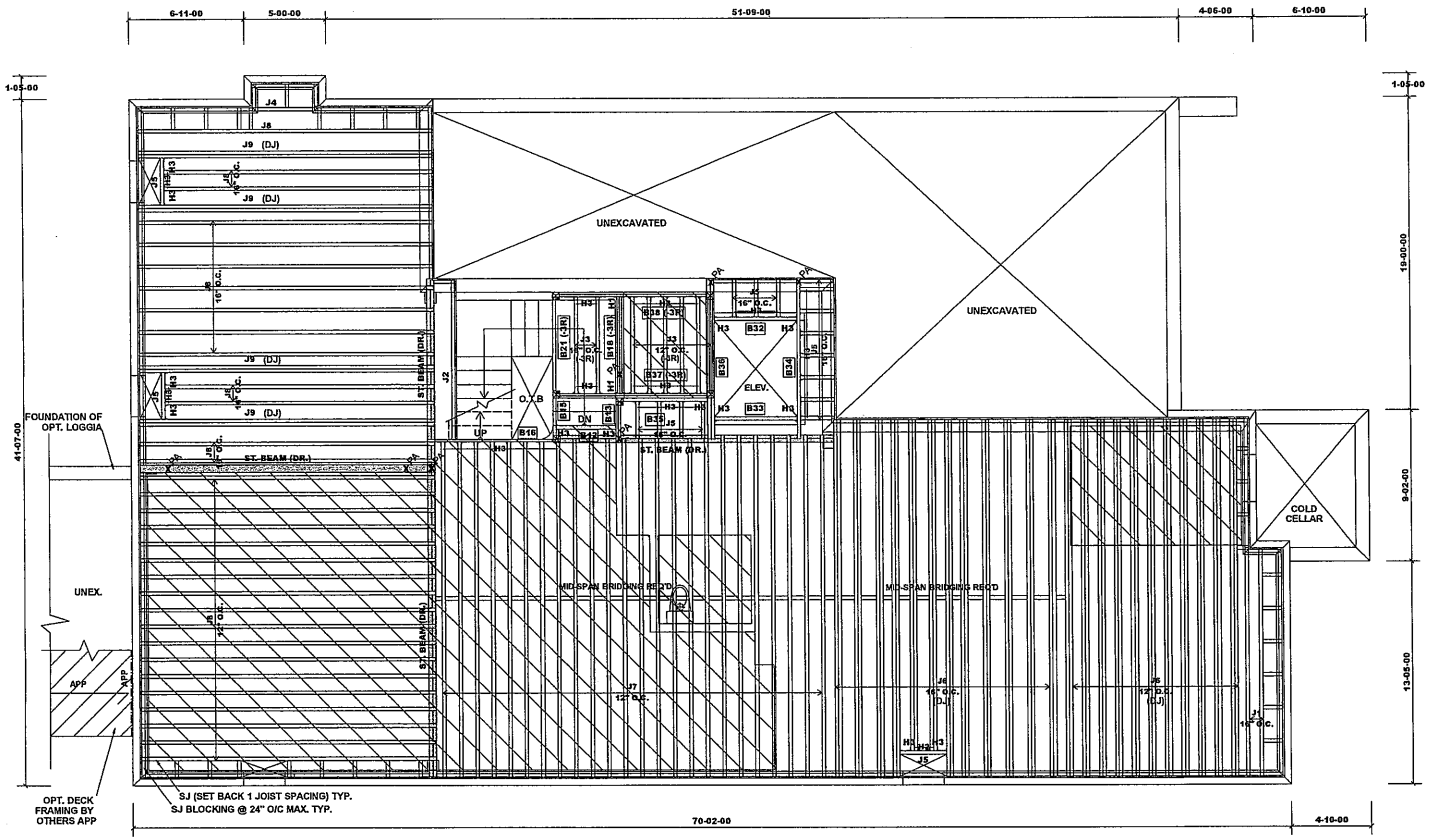
Builder: Gold Park Homes
Project: Pine Valley Ph2

Location: Vaughan, ON
Date: May. 06, 2022

Designer: TL
Sheet: 1 of 36

Alpa Roof Trusses Inc.
Stouffville, Ontario

Salesperson: Derek F.
Home Lumber Inc.



Products				
PlotID	Length	Product	Plies	Net Qty
B12	4-00-00	11 7/8" NI-20	1	1
B13	3-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B15	3-00-00	11 7/8" NI-20	1	1
B16	9-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	1	1
B18 (-3R)	6-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B21 (-3R)	7-00-00	11 7/8" NI-20	1	1
B32	5-00-00	11 7/8" NI-20	1	1
B33	5-00-00	11 7/8" NI-20	1	1
B34	10-00-00	11 7/8" NI-20	1	1
B35	6-00-00	11 7/8" NI-20	1	1
B36	10-00-00	11 7/8" NI-20	2	2
B37 (-3R)	10-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B38 (-3R)	10-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
J1	14-00-00	11 7/8" NI-20	1	2
J2	10-00-00	11 7/8" NI-20	1	1
J3	6-00-00	11 7/8" NI-20	1	8
J4	4-00-00	11 7/8" NI-20	1	1
J5	3-00-00	11 7/8" NI-20	1	18
J6	22-00-00	11 7/8" NI-40x	2	46
J7	21-00-00	11 7/8" NI-40x	1	24
J8	18-00-00	11 7/8" NI-40x	1	32
J9	18-00-00	11 7/8" NI-40x	2	8
Ca1	224-00-00	1 1/8" x 11 7/8" Rim Board	1	1
Bk1	105-00-00	11 7/8" NI-20	1	1

Connector Summary			
PlotID	Qty	Manuf	Product
H1	2		HGUS410
H2	2		HU312-2
H3	54		LT251188

GROUND FLOOR FRAMING
UNIT 5013 - THE RIVERVIEW
ELEVATION A
+ W/ W.O.D. CONDITION
W/ ELEVATOR

FLOOR LOADING
LIVE LOAD : 40 PSF
DEAD LOAD : 15 PSF
DEAD LOAD (TILE): 20 PSF

HATCH LEGEND
Ceramic Tile
Conv Framed

APP - AS PER PLAN
BBO - BEAM BY OTHERS
PA - POST ABOVE
O.T.B - OPEN TO BELOW
GT - GIRDER TRUSS
RT - ROOF TRUSS

RIMBOARD
1-1/8" X 11-7/8" O.S.B

SUBFLOOR: 3/4" NAILED & GLUED*

Blocking panels are required over all interior supports.
Squash blocks are required under concentrated loads.

Ceramic Tile Application as per O.B.C. 9.30.6

Provide I-Joist blocking between cantilevered joists (along bearing) and rimboard closure at ends.

Do not scale - refer to architectural plans for dimensions.

JT/PL: 45147/116460
LI: 343077*

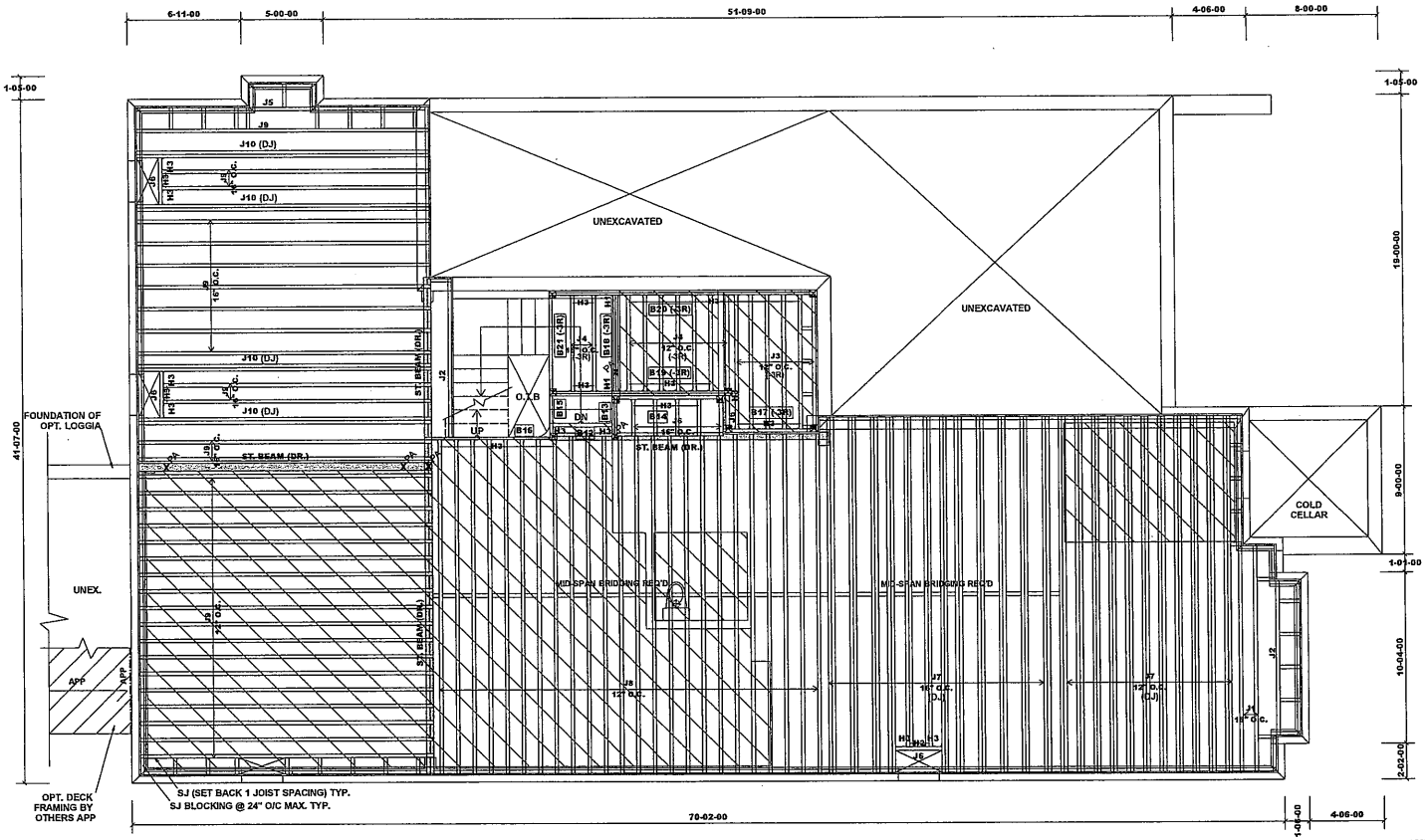
Builder: Gold Park Homes
Project: Pine Valley Ph2

Location: Vaughan, ON
Date: May. 06, 2022

Designer: TL
Sheet: 2 of 36

Alpa Roof Trusses Inc.
Stouffville, Ontario

Salesperson: Derek F.
Home Lumber Inc.



Products				
PlotID	Length	Product	Plies	Net Qty
B12	4-00-00	11 7/8" NI-20	1	1
B13	3-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B14	7-00-00	11 7/8" NI-20	1	1
B15	3-00-00	11 7/8" NI-20	1	1
B16	9-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	1	1
B17 (-3R)	6-00-00	11 7/8" NI-20	1	1
B18 (-3R)	6-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B19 (-3R)	11-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B20 (-3R)	17-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B21 (-3R)	7-00-00	11 7/8" NI-20	1	1
J1	14-00-00	11 7/8" NI-20	1	2
J2	10-00-00	11 7/8" NI-20	1	2
J3	9-00-00	11 7/8" NI-20	1	6
J4	6-00-00	11 7/8" NI-20	1	9
J5	4-00-00	11 7/8" NI-20	1	1
J8	3-00-00	11 7/8" NI-20	1	8
J7	22-00-00	11 7/8" NI-40x	2	46
J8	21-00-00	11 7/8" NI-40x	1	24
J9	18-00-00	11 7/8" NI-40x	1	32
J10	18-00-00	11 7/8" NI-40x	2	8
Ca1	225-00-00	1 1/8" x 11 7/8" Rim Board	1	1
Bk1	106-00-00	11 7/8" NI-20	1	1

Connector Summary			
PlotID	Qty	Manuf	Product
H1	2		HGUS410
H2	2		HUS12-2
H3	53		LT251188

GROUND FLOOR FRAMING
UNIT 5013 - THE RIVERVIEW
ELEVATION B
+ W/ W.O.D. CONDITION

FLOOR LOADING
LIVE LOAD : 40 PSF
DEAD LOAD : 15 PSF
DEAD LOAD (TILE): 20 PSF

HATCH LEGEND
Ceramic Tile
Conv Framed

APP - AS PER PLAN
BBO - BEAM BY OTHERS
PA - POST ABOVE
O.T.B - OPEN TO BELOW
GT - GIRDER TRUSS
RT - ROOF TRUSS

RIMBOARD
1-1/8" X 11-7/8" O.S.B

SUBFLOOR: 3/4" NAILED & GLUED*

Blocking panels are required over all interior supports.
Squash blocks are required under concentrated loads.

Ceramic Tile Application as per O.B.C. 9.30.6

Provide I-Joist blocking between cantilevered joists (along bearing) and rimboard closure at ends.

Do not scale - refer to architectural plans for dimensions.

JT/PL: 45147/116460
LI: 343077*

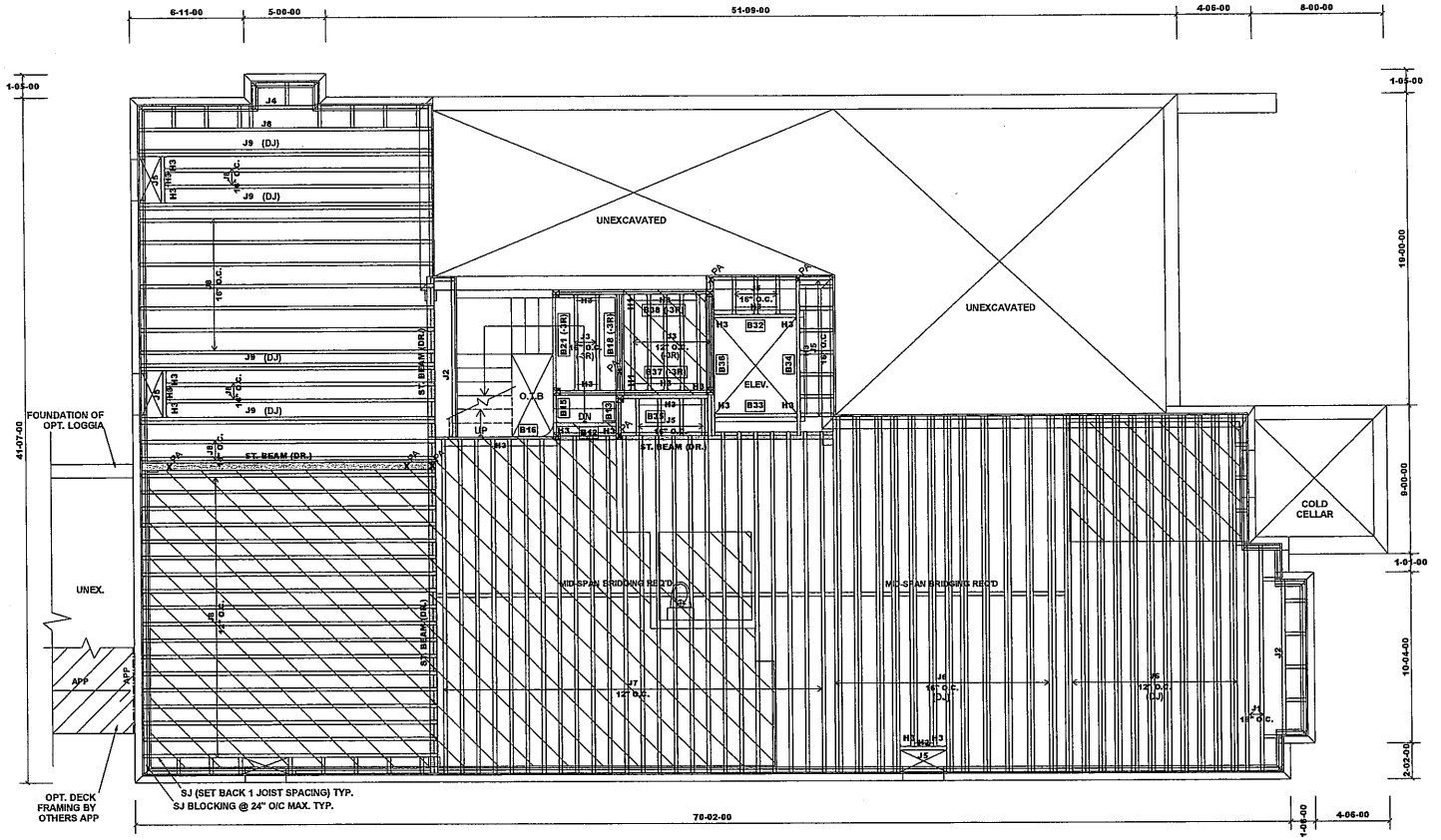
Builder: Gold Park Homes
Project: Pine Valley Ph2

Location: Vaughan, ON
Date: May. 06, 2022

Designer: TL
Sheet: 3 of 36

Alpa Roof Trusses Inc.
Stouffville, Ontario

Salesperson: Derek F.
Home Lumber Inc.



Products				
PlotID	Length	Product	Plies	Net Qty
B12	4-00-00	11 7/8" NI-20	1	1
B13	3-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B15	3-00-00	11 7/8" NI-20	1	1
B16	9-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	1	1
B18 (-3R)	6-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B21 (-3R)	7-00-00	11 7/8" NI-20	1	1
B32	5-00-00	11 7/8" NI-20	1	1
B33	5-00-00	11 7/8" NI-20	1	1
B34	10-00-00	11 7/8" NI-20	1	1
B35	6-00-00	11 7/8" NI-20	1	1
B36	10-00-00	11 7/8" NI-20	2	2
B37 (-3R)	10-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B38 (-3R)	10-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
J1	14-00-00	11 7/8" NI-20	1	2
J2	10-00-00	11 7/8" NI-20	1	2
J3	6-00-00	11 7/8" NI-20	1	8
J4	4-00-00	11 7/8" NI-20	1	1
J5	3-00-00	11 7/8" NI-20	1	18
J6	22-00-00	11 7/8" NI-40x	2	46
J7	21-00-00	11 7/8" NI-40x	1	24
J8	18-00-00	11 7/8" NI-40x	1	32
J9	18-00-00	11 7/8" NI-40x	2	8
Ca1	227-00-00	1 1/8" x 11 7/8" Rim Board	1	1
Bk1	107-00-00	11 7/8" NI-20	1	1

Connector Summary			
PlotID	Qty	Manuf	Product
H1	2		HGUS410
H2	2		HU312-2
H3	54		LT251188

GROUND FLOOR FRAMING
UNIT 5013 - THE RIVERVIEW
ELEVATION B
+ W/ W.O.D. CONDITION
W/ ELEVATOR

FLOOR LOADING
LIVE LOAD : 40 PSF
DEAD LOAD : 15 PSF
DEAD LOAD (TILE): 20 PSF

HATCH LEGEND
- Ceramic Tile
- Conv Framed

APP - AS PER PLAN
BBO - BEAM BY OTHERS
PA - POST ABOVE
O.T.B - OPEN TO BELOW
GT - GIRDER TRUSS
RT - ROOF TRUSS

RIMBOARD
1-1/8" X 11-7/8" O.S.B

SUBFLOOR: 3/4" NAILED & GLUED*

Blocking panels are required over all interior supports.
Squash blocks are required under concentrated loads.

Ceramic Tile Application as per O.B.C. 9.30.6

Provide I-Joist blocking between cantilevered joists (along bearing) and rimboard closure at ends.

Do not scale - refer to architectural plans for dimensions.

JT/PL: 45147/116460
LI: 343077*

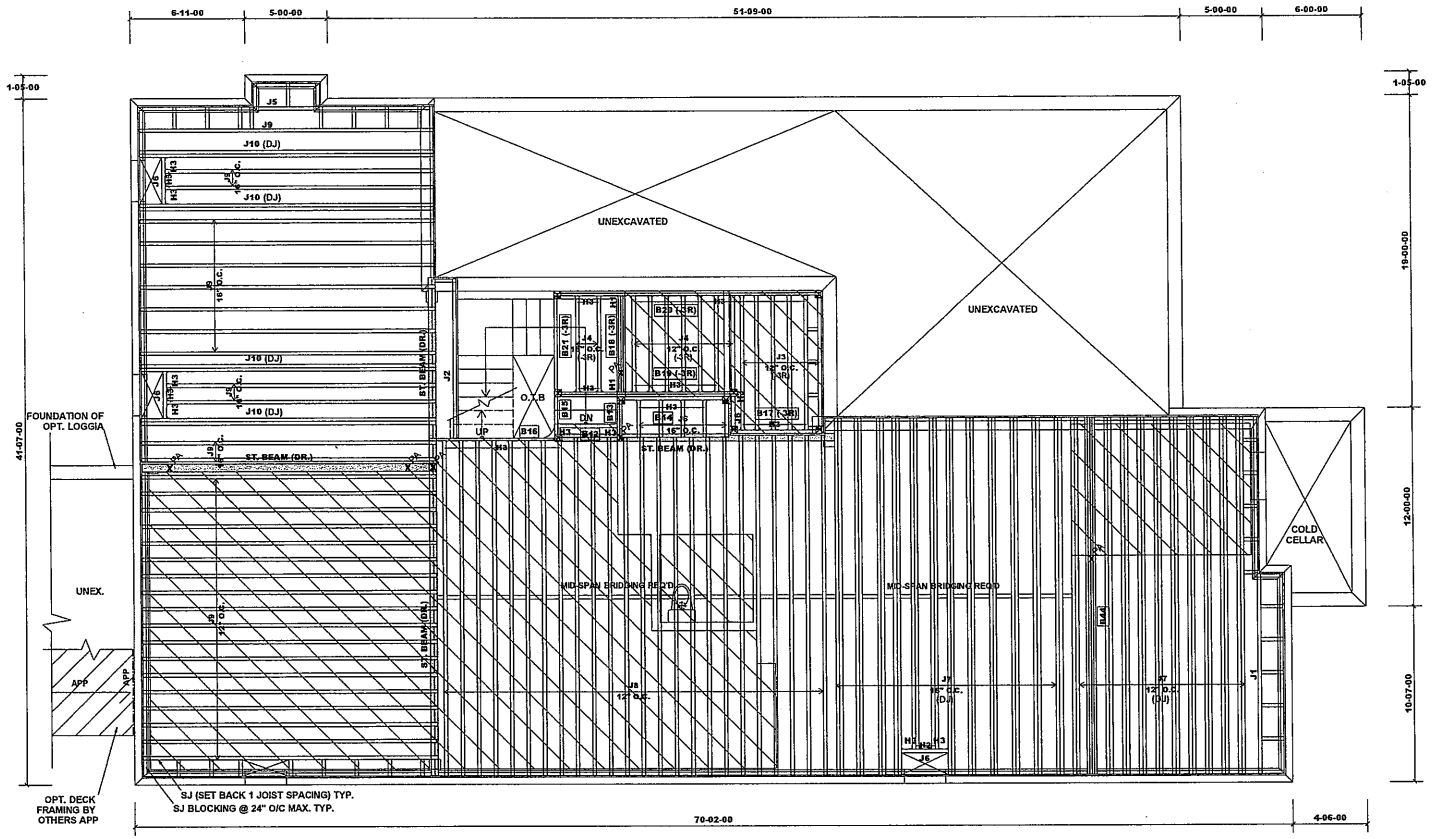
Builder: Gold Park Homes
Project: Pine Valley Ph2

Location: Vaughan, ON
Date: May. 06, 2022

Designer: TL
Sheet: 4 of 36

Alpa Roof Trusses Inc.
Stouffville, Ontario

Salesperson: Derek F.
Home Lumber Inc.



Products				
PlotID	Length	Product	Plies	Net Qty
B12	4-00-00	11 7/8" NI-20	1	1
B13	3-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B14	7-00-00	11 7/8" NI-20	1	1
B15	3-00-00	11 7/8" NI-20	1	1
B16	9-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	1	1
B17 (-3R)	6-00-00	11 7/8" NI-20	1	1
B18 (-3R)	6-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B19 (-3R)	11-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B20 (-3R)	17-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B21 (-3R)	7-00-00	11 7/8" NI-20	1	1
B44	22-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	4	4
J1	13-00-00	11 7/8" NI-20	1	1
J2	10-00-00	11 7/8" NI-20	1	1
J3	9-00-00	11 7/8" NI-20	1	6
J4	6-00-00	11 7/8" NI-20	1	9
J5	4-00-00	11 7/8" NI-20	1	1
J6	3-00-00	11 7/8" NI-20	1	9
J7	22-00-00	11 7/8" NI-40x	2	44
J8	21-00-00	11 7/8" NI-40x	1	24
J9	18-00-00	11 7/8" NI-40x	1	32
J10	18-00-00	11 7/8" NI-40x	2	8
Ca1	222-00-00	1 1/8" x 11 7/8" Rim Board	1	1
Bk1	105-00-00	11 7/8" NI-20	1	1

Connector Summary			
PlotID	Qty	Manuf	Product
H1	2		HGUS410
H2	2		HU312-2
H3	53		LT251188

GROUND FLOOR FRAMING
UNIT 5013 - THE RIVERVIEW
ELEVATION C
+ W/ W.O.D. CONDITION

FLOOR LOADING
LIVE LOAD : 40 PSF
DEAD LOAD : 15 PSF
DEAD LOAD (TILE): 20 PSF

HATCH LEGEND
- Ceramic Tile
- Conv Framed

APP - AS PER PLAN
BBO - BEAM BY OTHERS
PA - POST ABOVE
O.T.B - OPEN TO BELOW
GT - GIRDER TRUSS
RT - ROOF TRUSS

RIMBOARD
1-1/8" X 11-7/8" O.S.B

SUBFLOOR: 3/4" NAILED & GLUED*

Blocking panels are required over all interior supports.
Squash blocks are required under concentrated loads.

Ceramic Tile Application as per O.B.C. 9.30.6

Provide I-Joist blocking between cantilevered joists (along bearing) and rimboard closure at ends.

Do not scale - refer to architectural plans for dimensions.

JT/PL: 45147/116460
LI: 343077*

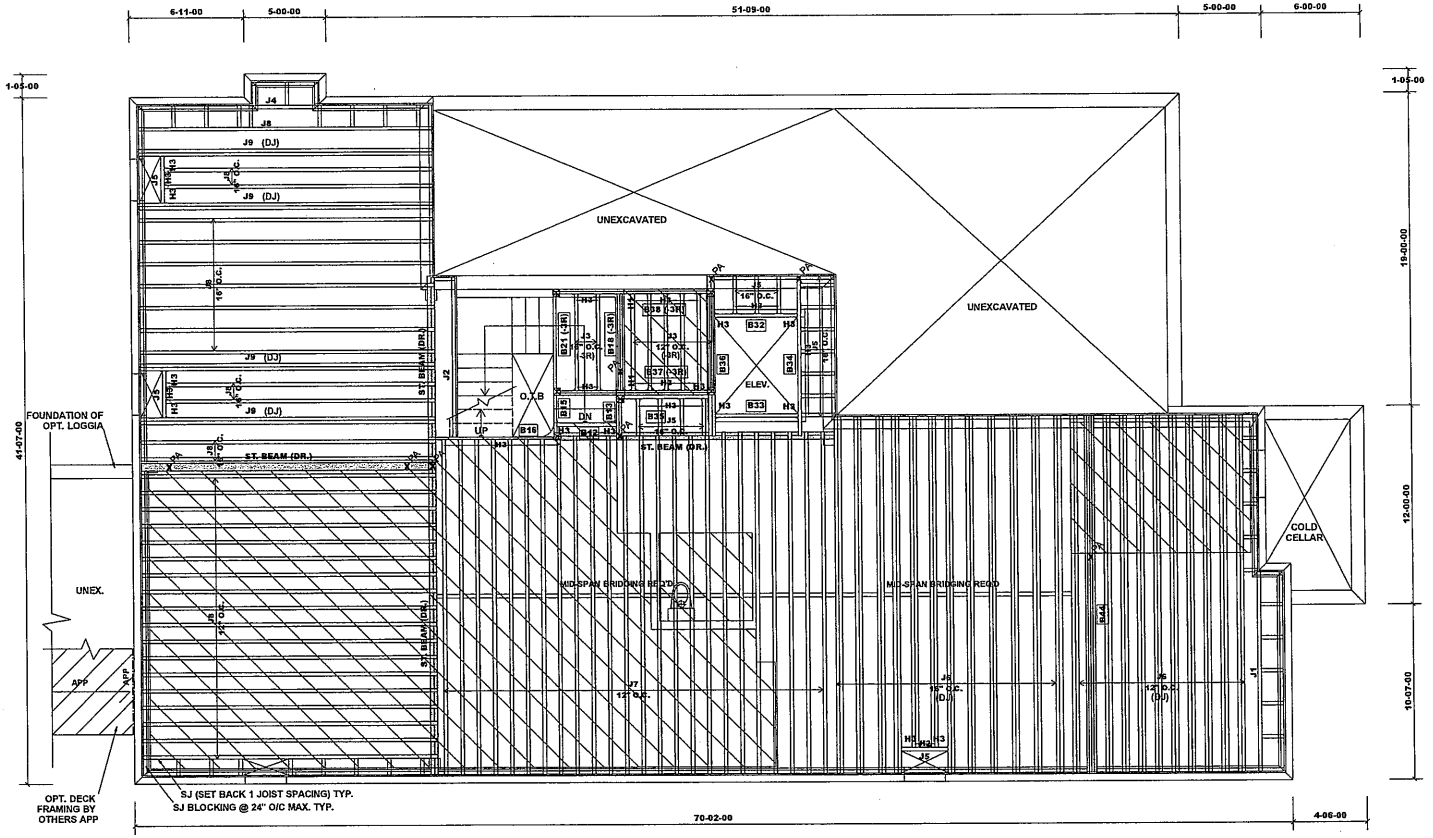
Builder: Gold Park Homes
Project: Pine Valley Ph2

Location: Vaughan, ON
Date: May. 06, 2022

Designer: TL
Sheet: 5 of 36

Alpa Roof Trusses Inc.
Stouffville, Ontario

Salesperson: Derek F.
Home Lumber Inc.



Products				
PlotID	Length	Product	Plies	Net Qty
B12	4-00-00	11 7/8" NI-20	1	1
B13	3-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B15	3-00-00	11 7/8" NI-20	1	1
B16	9-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	1	1
B18 (-3R)	6-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B21 (-3R)	7-00-00	11 7/8" NI-20	1	1
B32	5-00-00	11 7/8" NI-20	1	1
B33	5-00-00	11 7/8" NI-20	1	1
B34	10-00-00	11 7/8" NI-20	1	1
B35	6-00-00	11 7/8" NI-20	1	1
B36	10-00-00	11 7/8" NI-20	2	2
B37 (-3R)	10-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B38 (-3R)	10-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B44	22-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	4	4
J1	13-00-00	11 7/8" NI-20	1	1
J2	10-00-00	11 7/8" NI-20	1	1
J3	6-00-00	11 7/8" NI-20	1	8
J4	4-00-00	11 7/8" NI-20	1	1
J5	3-00-00	11 7/8" NI-20	1	18
J6	22-00-00	11 7/8" NI-40x	2	44
J7	21-00-00	11 7/8" NI-40x	1	24
J8	18-00-00	11 7/8" NI-40x	1	32
J9	18-00-00	11 7/8" NI-40x	2	8
Ca1	224-00-00	1 1/8" x 11 7/8" Rim Board	1	1
Bk1	107-00-00	11 7/8" NI-20	1	1

Connector Summary			
PlotID	Qty	Manuf	Product
H1	2		HGUS410
H2	2		HU312-2
H3	54		LT251188

GROUND FLOOR FRAMING
UNIT 5013 - THE RIVERVIEW
ELEVATION C
+ W/ W.O.D. CONDITION
W/ ELEVATOR

FLOOR LOADING
LIVE LOAD : 40 PSF
DEAD LOAD : 15 PSF
DEAD LOAD (TILE): 20 PSF

HATCH LEGEND
Ceramic Tile
Conv Framed

APP - AS PER PLAN
BBO - BEAM BY OTHERS
PA - POST ABOVE
O.T.B - OPEN TO BELOW
GT - GIRDER TRUSS
RT - ROOF TRUSS

RIMBOARD
1-1/8" X 11-7/8" O.S.B

SUBFLOOR: 3/4" NAILED & GLUED*

Blocking panels are required over all interior supports.
Squash blocks are required under concentrated loads.

Ceramic Tile Application as per O.B.C. 9.30.6

Provide I-Joist blocking between cantilevered joists (along bearing) and rimboard closure at ends.

Do not scale - refer to architectural plans for dimensions.

JT/PL: 45147/116460
LI: 343077*

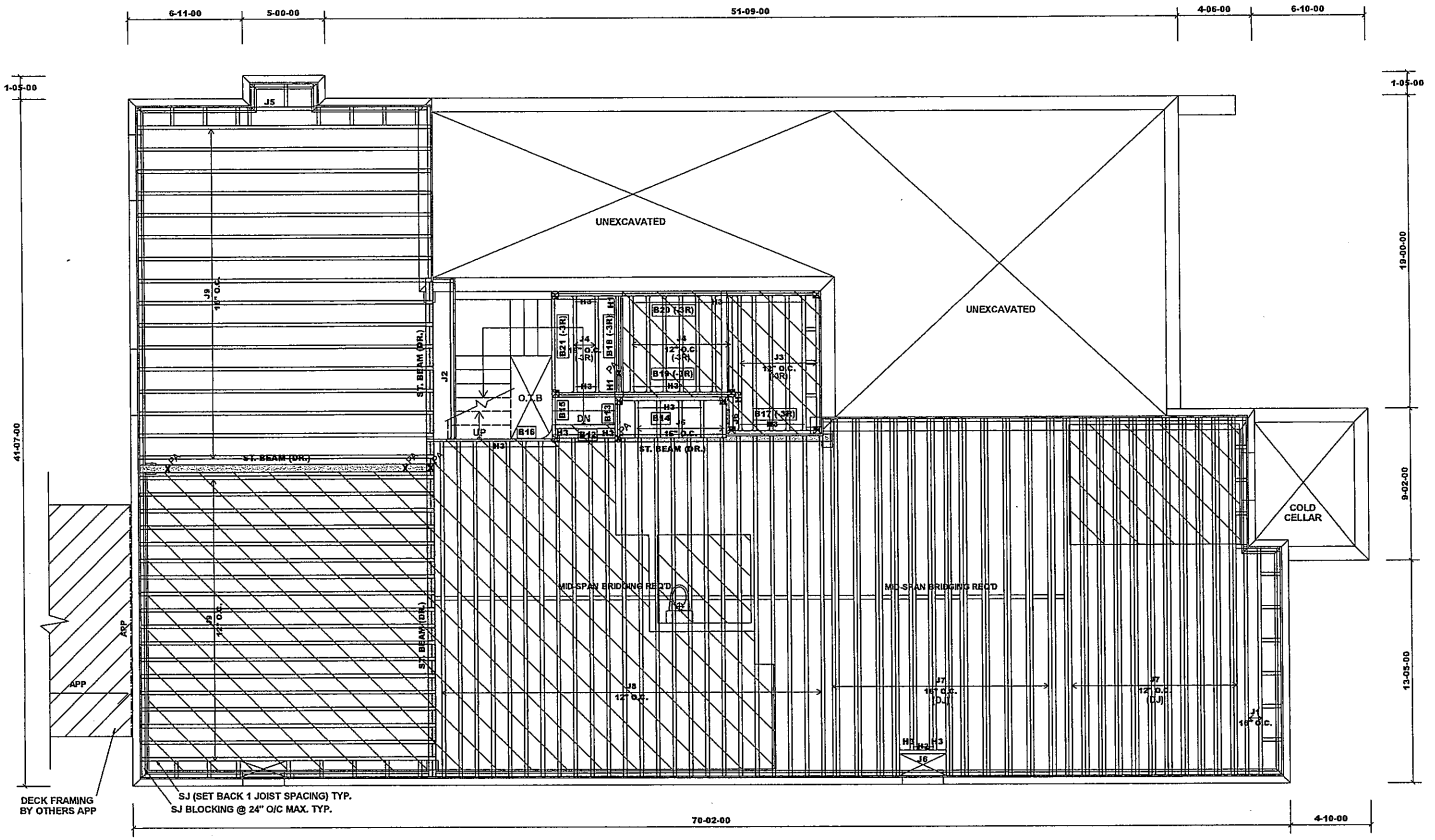
Builder: Gold Park Homes
Project: Pine Valley Ph2

Location: Vaughan, ON
Date: May. 06, 2022

Designer: TL
Sheet: 6 of 36

Alpa Roof Trusses Inc.
Stouffville, Ontario

Salesperson: Derek F.
Home Lumber Inc.



Products				
PlotID	Length	Product	Plies	Net Qty
B12	4-00-00	11 7/8" NI-20	1	1
B13	3-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B14	7-00-00	11 7/8" NI-20	1	1
B15	3-00-00	11 7/8" NI-20	1	1
B16	9-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	1	1
B17 (-3R)	6-00-00	11 7/8" NI-20	1	1
B18 (-3R)	6-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B19 (-3R)	11-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B20 (-3R)	17-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B21 (-3R)	7-00-00	11 7/8" NI-20	1	1
J1	14-00-00	11 7/8" NI-20	1	2
J2	10-00-00	11 7/8" NI-20	1	1
J3	9-00-00	11 7/8" NI-20	1	6
J4	6-00-00	11 7/8" NI-20	1	9
J5	4-00-00	11 7/8" NI-20	1	1
J6	3-00-00	11 7/8" NI-20	1	7
J7	22-00-00	11 7/8" NI-40x	2	46
J8	21-00-00	11 7/8" NI-40x	1	24
J9	18-00-00	11 7/8" NI-40x	1	34
Ca1	222-00-00	1 1/8" x 11 7/8" Rim Board	1	1
Bk1	102-00-00	11 7/8" NI-20	1	1

Connector Summary			
PlotID	Qty	Manuf	Product
H1	2		HGUS410
H2	2		HU312-2
H3	45		LT251188

GROUND FLOOR FRAMING
UNIT 5013 - THE RIVERVIEW
ELEVATION A
W/ L.O.D. CONDITION

FLOOR LOADING
LIVE LOAD : 40 PSF
DEAD LOAD : 15 PSF
DEAD LOAD (TILE): 20 PSF

HATCH LEGEND
Ceramic Tile
Conv Framed

APP - AS PER PLAN
BBO - BEAM BY OTHERS
PA - POST ABOVE
O.T.B - OPEN TO BELOW
GT - GIRDER TRUSS
RT - ROOF TRUSS

RIMBOARD
1-1/8" X 11-7/8" O.S.B

SUBFLOOR: 3/4" NAILED & GLUED*

Blocking panels are required over all interior supports.
Squash blocks are required under concentrated loads.

Ceramic Tile Application as per O.B.C. 9.30.6

Provide I-Joist blocking between cantilevered joists (along bearing) and rimboard closure at ends.

Do not scale - refer to architectural plans for dimensions.

JT/PL: 45147/116460
LI: 343077*

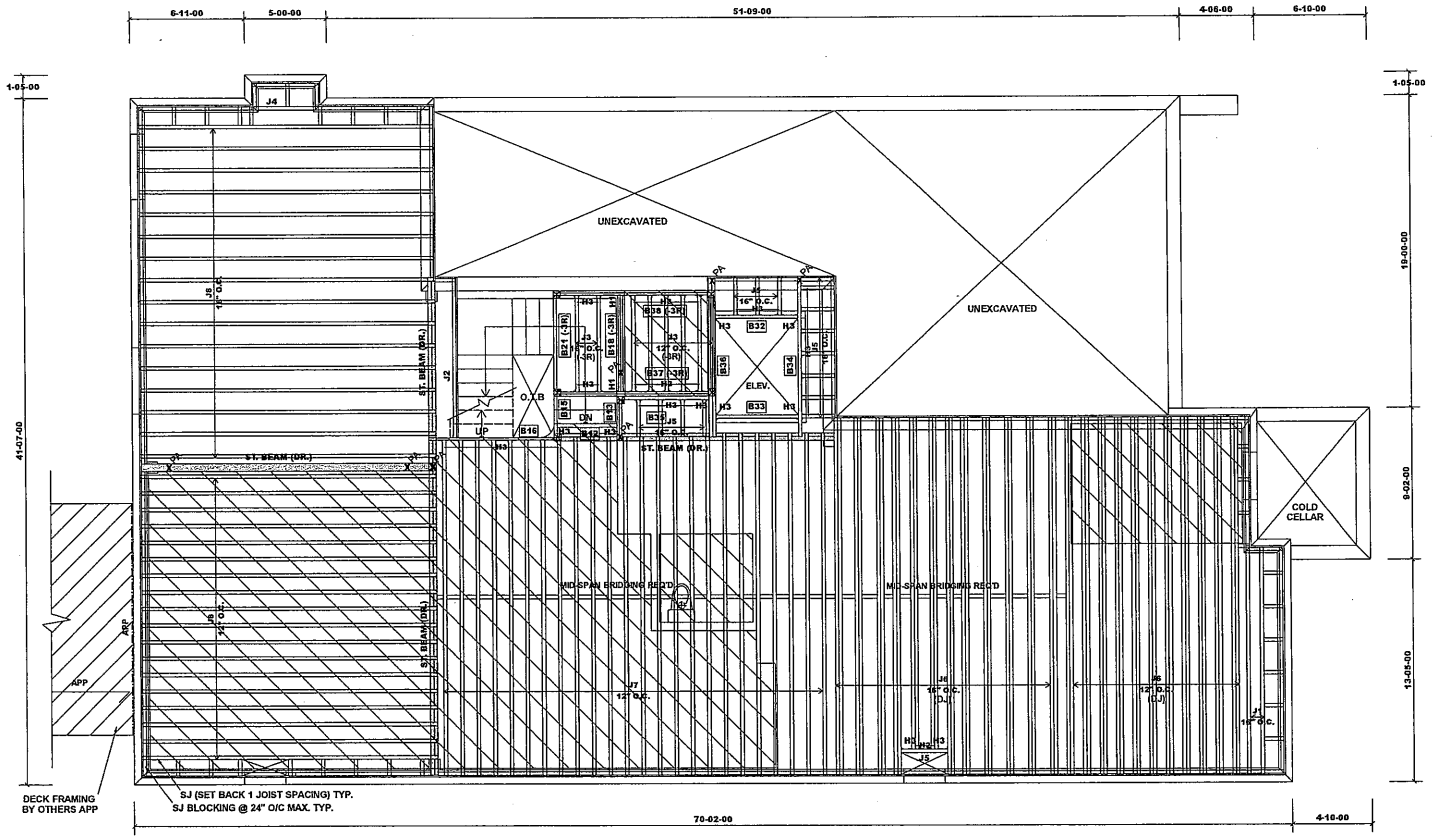
Builder: Gold Park Homes
Project: Pine Valley Ph2

Location: Vaughan, ON
Date: May. 06, 2022

Designer: TL
Sheet: 7 of 36

Alpa Roof Trusses Inc.
Stouffville, Ontario

Salesperson: Derek F.
Home Lumber Inc.



Products				
PlotID	Length	Product	Plies	Net Qty
B12	4-00-00	11 7/8" NI-20	1	1
B13	3-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B15	3-00-00	11 7/8" NI-20	1	1
B16	9-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	1	1
B18 (-3R)	6-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B21 (-3R)	7-00-00	11 7/8" NI-20	1	1
B32	5-00-00	11 7/8" NI-20	1	1
B33	5-00-00	11 7/8" NI-20	1	1
B34	10-00-00	11 7/8" NI-20	1	1
B35	6-00-00	11 7/8" NI-20	1	1
B36	10-00-00	11 7/8" NI-20	2	2
B37 (-3R)	10-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B38 (-3R)	10-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
J1	14-00-00	11 7/8" NI-20	1	2
J2	10-00-00	11 7/8" NI-20	1	1
J3	6-00-00	11 7/8" NI-20	1	8
J4	4-00-00	11 7/8" NI-20	1	1
J5	3-00-00	11 7/8" NI-20	1	16
J6	22-00-00	11 7/8" NI-40x	2	45
J7	21-00-00	11 7/8" NI-40x	1	24
J8	18-00-00	11 7/8" NI-40x	1	34
Ca1	224-00-00	1 1/8" x 11 7/8" Rim Board	1	1
Blk1	103-00-00	11 7/8" NI-20	1	1

Connector Summary			
PlotID	Qty	Manuf	Product
H1	2		HGUS410
H2	2		HU312-2
H3	46		LT251188

GROUND FLOOR FRAMING
UNIT 5013 - THE RIVERVIEW
ELEVATION A
W/ L.O.D. CONDITION
W/ ELEVATOR

FLOOR LOADING
LIVE LOAD : 40 PSF
DEAD LOAD : 15 PSF
DEAD LOAD (TILE): 20 PSF

HATCH LEGEND
- Ceramic Tile
- Conv Framed

APP - AS PER PLAN
BBO - BEAM BY OTHERS
PA - POST ABOVE
O.T.B - OPEN TO BELOW
GT - GIRDER TRUSS
RT - ROOF TRUSS

RIMBOARD
1-1/8" X 11-7/8" O.S.B

SUBFLOOR: 3/4" NAILED & GLUED*

Blocking panels are required over all interior supports.
Squash blocks are required under concentrated loads.

Ceramic Tile Application as per O.B.C. 9.30.6

Provide I-Joist blocking between cantilevered joists (along bearing) and rimboard closure at ends.

Do not scale - refer to architectural plans for dimensions.

JT/PL: 45147/116460
LI: 343077*

Builder: Gold Park Homes
Project: Pine Valley Ph2

Location: Vaughan, ON
Date: May. 06, 2022

Designer: TL
Sheet: 8 of 36

Alpa Roof Trusses Inc.
Stouffville, Ontario

Salesperson: Derek F.
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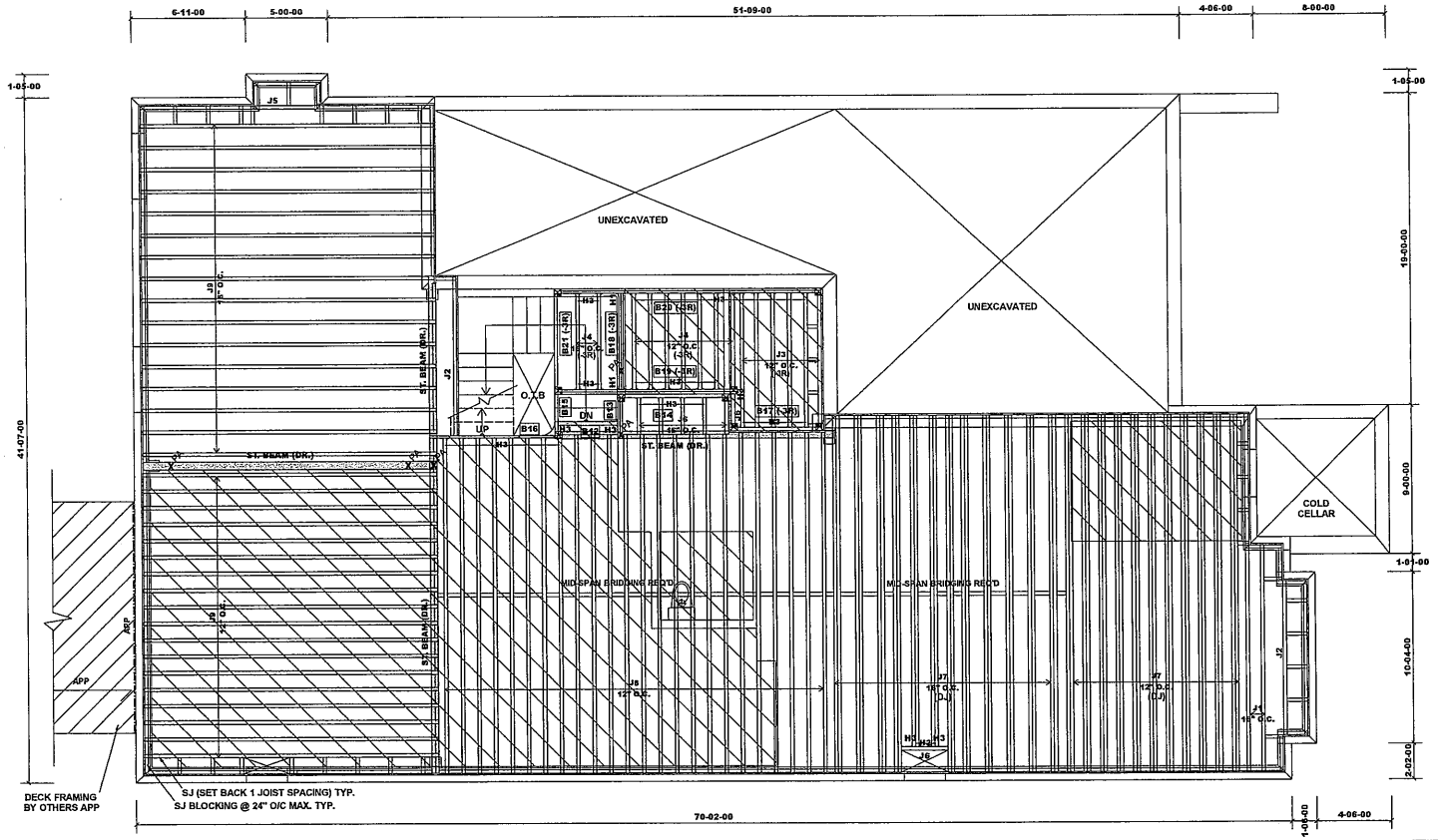


Products				
PlotID	Length	Product	Plies	Net Qty
B12	4-00-00	11 7/8" NI-20	1	1
B13	3-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B14	7-00-00	11 7/8" NI-20	1	1
B15	3-00-00	11 7/8" NI-20	1	1
B16	6-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	1	1
B17 (-3R)	6-00-00	11 7/8" NI-20	1	1
B18 (-3R)	6-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B19 (-3R)	11-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B20 (-3R)	17-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B21 (-3R)	7-00-00	11 7/8" NI-20	1	1
J1	14-00-00	11 7/8" NI-20	1	2
J2	10-00-00	11 7/8" NI-20	1	2
J3	9-00-00	11 7/8" NI-20	1	6
J4	6-00-00	11 7/8" NI-20	1	9
J5	4-00-00	11 7/8" NI-20	1	1
J6	3-00-00	11 7/8" NI-20	1	7
J7	22-00-00	11 7/8" NI-40x	2	46
J8	21-00-00	11 7/8" NI-40x	1	24
J9	18-00-00	11 7/8" NI-40x	1	34
Ca1	225-00-00	1 1/8" x 11 7/8" Rim Board	1	1
Bk1	104-00-00	11 7/8" NI-20	1	1

Connector Summary			
PlotID	Qty	Manuf	Product
H1	2		HGUS410
H2	2		HU312-2
H3	45		LT251188

GROUND FLOOR FRAMING
UNIT 5013 - THE RIVERVIEW
ELEVATION B
W/ L.O.D. CONDITION

FLOOR LOADING
LIVE LOAD : 40 PSF
DEAD LOAD : 15 PSF
DEAD LOAD (TILE): 20 PSF

HATCH LEGEND
- Ceramic Tile
- Conv Framed

APP - AS PER PLAN
BBO - BEAM BY OTHERS
PA - POST ABOVE
O.T.B - OPEN TO BELOW
GT - GIRDER TRUSS
RT - ROOF TRUSS

RIMBOARD
1-1/8" X 11-7/8" O.S.B

SUBFLOOR: 3/4" NAILED & GLUED*

Blocking panels are required over all interior supports.
Squash blocks are required under concentrated loads.

Ceramic Tile Application as per O.B.C. 9.30.6

Provide I-Joist blocking between cantilevered joists (along bearing) and rimboard closure at ends.

Do not scale - refer to architectural plans for dimensions.

JT/PL: 45147/116460
LI: 343077*

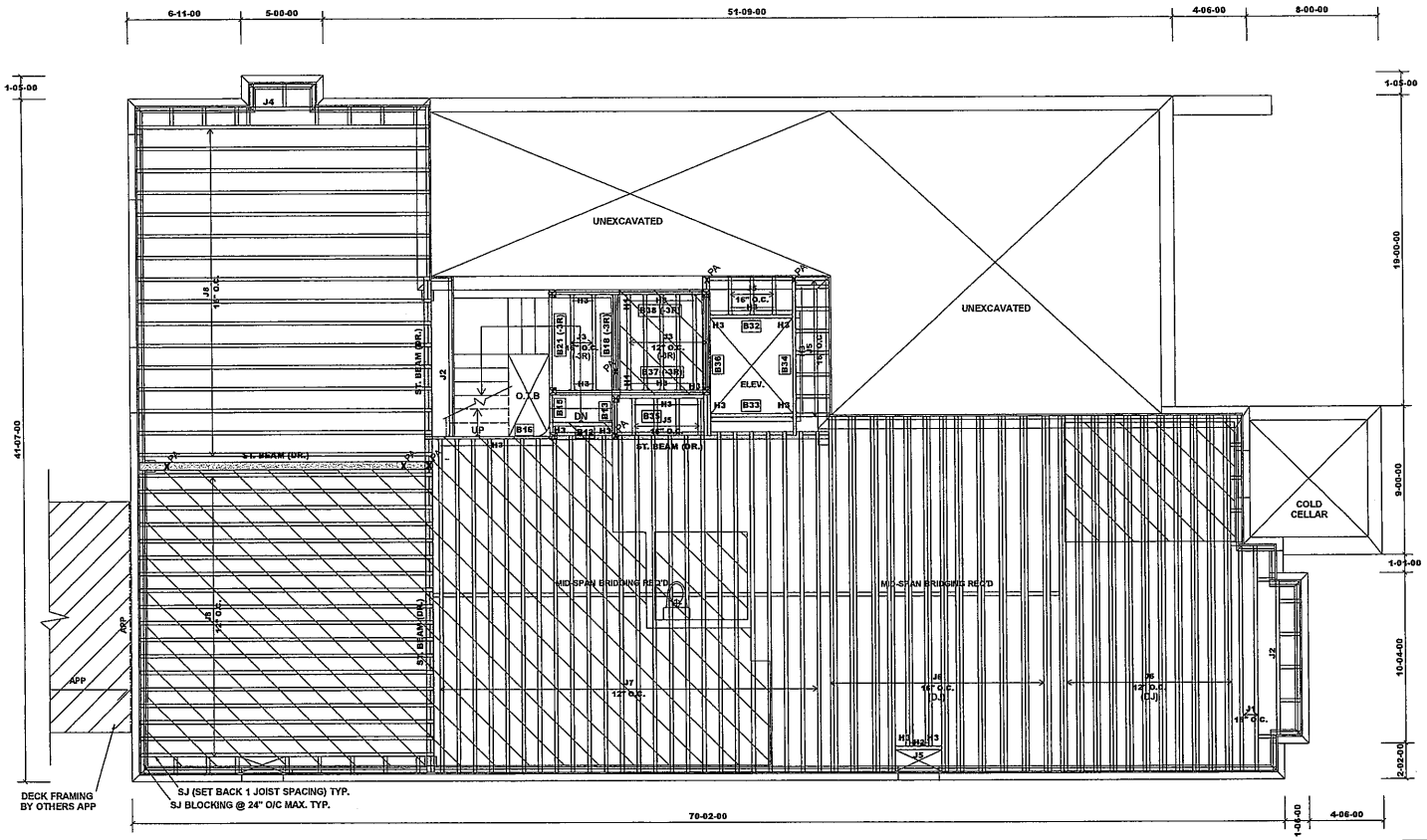
Builder: Gold Park Homes
Project: Pine Valley Ph2

Location: Vaughan, ON
Date: May. 06, 2022

Designer: TL
Sheet: 9 of 36

Alpa Roof Trusses Inc.
Stouffville, Ontario

Salesperson: Derek F.
Home Lumber Inc.



Products				
PlotID	Length	Product	Plies	Net Qty
B12	4-00-00	11 7/8" NI-20	1	1
B13	3-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B15	3-00-00	11 7/8" NI-20	1	1
B16	9-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	1	1
B18 (-3R)	6-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B21 (-3R)	7-00-00	11 7/8" NI-20	1	1
B32	5-00-00	11 7/8" NI-20	1	1
B33	5-00-00	11 7/8" NI-20	1	1
B34	10-00-00	11 7/8" NI-20	1	1
B35	6-00-00	11 7/8" NI-20	1	1
B36	10-00-00	11 7/8" NI-20	2	2
B37 (-3R)	10-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B38 (-3R)	10-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
J1	14-00-00	11 7/8" NI-20	1	2
J2	10-00-00	11 7/8" NI-20	1	2
J3	6-00-00	11 7/8" NI-20	1	8
J4	4-00-00	11 7/8" NI-20	1	1
J5	3-00-00	11 7/8" NI-20	1	16
J6	22-00-00	11 7/8" NI-40x	2	46
J7	21-00-00	11 7/8" NI-40x	1	24
J8	18-00-00	11 7/8" NI-40x	1	34
Ca1	227-00-00	1 1/8" x 11 7/8" Rim Board	1	1
Bk1	106-00-00	11 7/8" NI-20	1	1

Connector Summary			
PlotID	Qty	Manuf	Product
H1	2		HGUS410
H2	2		HU312-2
H3	46		LT2S1188

GROUND FLOOR FRAMING
UNIT 5013 - THE RIVERVIEW
ELEVATION B
W/ L.O.D. CONDITION
W/ ELEVATOR

FLOOR LOADING
LIVE LOAD : 40 PSF
DEAD LOAD : 15 PSF
DEAD LOAD (TILE): 20 PSF

HATCH LEGEND
Ceramic Tile
Conv Framed

APP - AS PER PLAN
BBO - BEAM BY OTHERS
PA - POST ABOVE
O.T.B - OPEN TO BELOW
GT - GIRDER TRUSS
RT - ROOF TRUSS

RIMBOARD
1-1/8" X 11-7/8" O.S.B

SUBFLOOR: 3/4" NAILED & GLUED*

Blocking panels are required over all interior supports.
Squash blocks are required under concentrated loads.

Ceramic Tile Application as per O.B.C. 9.30.6

Provide I-Joist blocking between cantilevered joists (along bearing) and rimboard closure at ends.

Do not scale - refer to architectural plans for dimensions.

JT/PL: 45147/116460
LI: 343077*

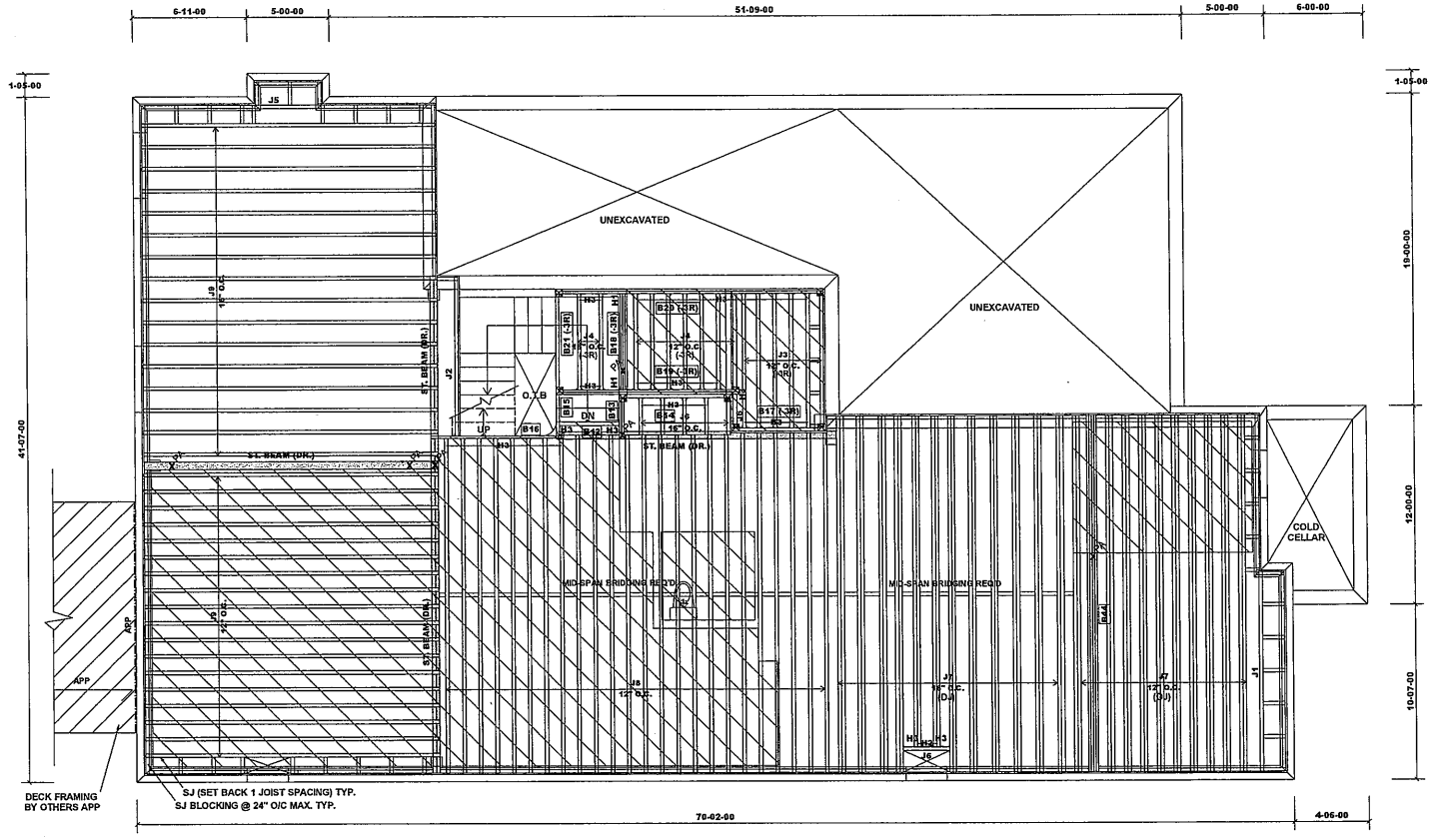
Builder: Gold Park Homes
Project: Pine Valley Ph2

Location: Vaughan, ON
Date: May. 06, 2022

Designer: TL
Sheet: 10 of 36

Alpa Roof Trusses Inc.
Stouffville, Ontario

Salesperson: Derek F.
Home Lumber Inc.



Products				
PlotID	Length	Product	Piles	Net Qty
B12	4-00-00	11 7/8" NI-20	1	1
B13	3-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B14	7-00-00	11 7/8" NI-20	1	1
B15	3-00-00	11 7/8" NI-20	1	1
B16	9-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	1	1
B17 (-3R)	6-00-00	11 7/8" NI-20	1	1
B18 (-3R)	6-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B19 (-3R)	11-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B20 (-3R)	17-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B21 (-3R)	7-00-00	11 7/8" NI-20	1	1
B44	22-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	4	4
J1	13-00-00	11 7/8" NI-20	1	1
J2	10-00-00	11 7/8" NI-20	1	1
J3	9-00-00	11 7/8" NI-20	1	6
J4	6-00-00	11 7/8" NI-20	1	9
J5	4-00-00	11 7/8" NI-20	1	1
J6	3-00-00	11 7/8" NI-20	1	7
J7	22-00-00	11 7/8" NI-40x	2	44
J8	21-00-00	11 7/8" NI-40x	1	24
J9	18-00-00	11 7/8" NI-40x	1	34
Ca1	222-00-00	1 1/8" x 11 7/8" Rim Board	1	1
Bk1	104-00-00	11 7/8" NI-20	1	1

Connector Summary			
PlotID	Qty	Manuf	Product
H1	2		HGUS410
H2	2		HUS12-2
H3	45		LT251188

GROUND FLOOR FRAMING
UNIT 5013 - THE RIVERVIEW
ELEVATION C
W/ L.O.D. CONDITION

FLOOR LOADING
LIVE LOAD : 40 PSF
DEAD LOAD : 15 PSF
DEAD LOAD (TILE): 20 PSF

HATCH LEGEND
- Ceramic Tile
- Conv Framed

APP - AS PER PLAN
BBO - BEAM BY OTHERS
PA - POST ABOVE
O.T.B - OPEN TO BELOW
GT - GIRDER TRUSS
RT - ROOF TRUSS

RIMBOARD
1-1/8" X 11-7/8" O.S.B

SUBFLOOR: 3/4" NAILED & GLUED*

Blocking panels are required over all interior supports.
Squash blocks are required under concentrated loads.

Ceramic Tile Application as per O.B.C. 9.30.6

Provide I-Joist blocking between cantilevered joists (along bearing) and rimboard closure at ends.

Do not scale - refer to architectural plans for dimensions.

JT/PL: 45147/116460
LI: 343077*

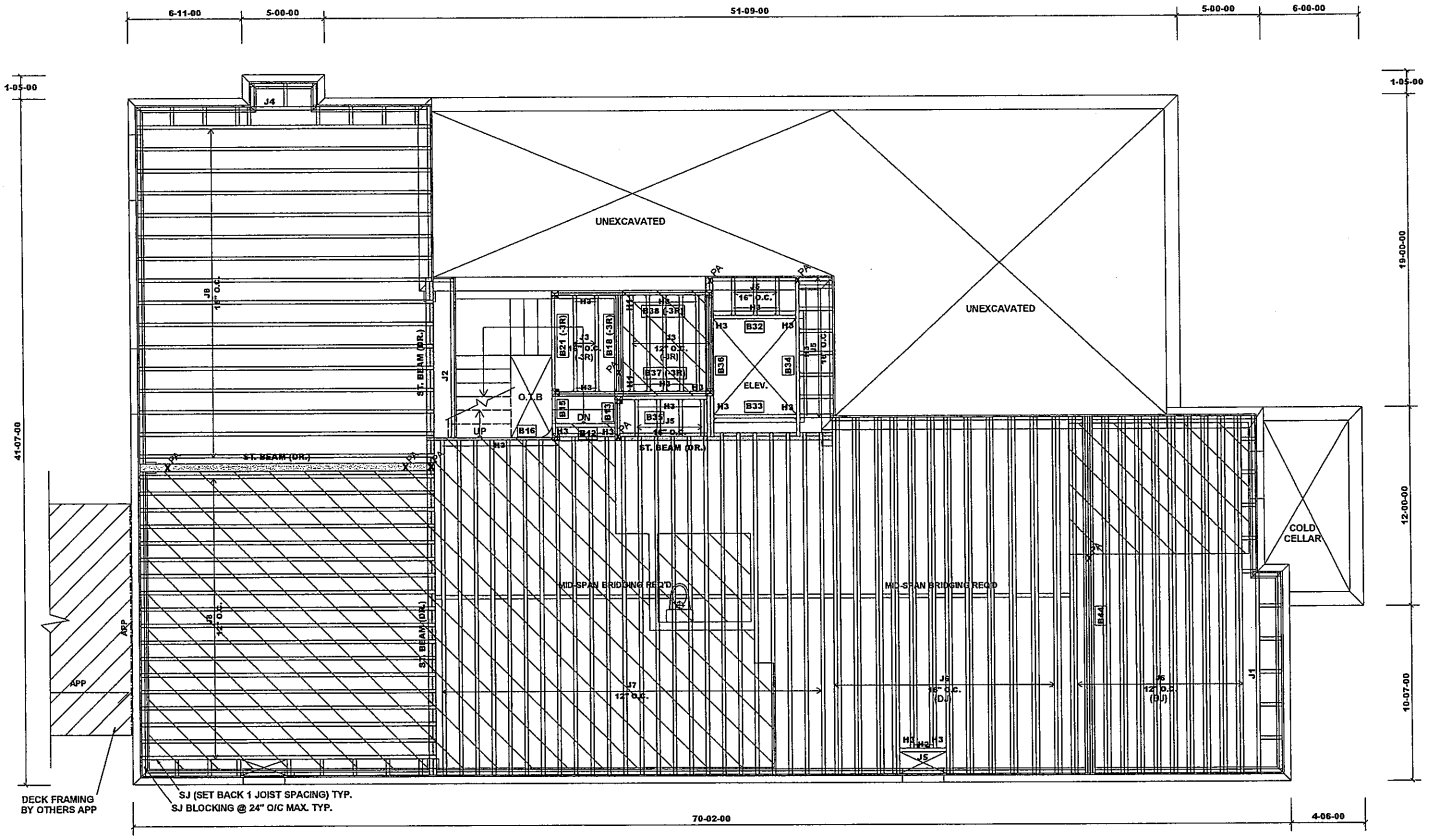
Builder: Gold Park Homes
Project: Pine Valley Ph2

Location: Vaughan, ON
Date: May. 06, 2022

Designer: TL
Sheet: 11 of 36

Alpa Roof Trusses Inc.
Stouffville, Ontario

Salesperson: Derek F.
Home Lumber Inc.



Products				
PlotID	Length	Product	Plies	Net Qty
B12	4-00-00	11 7/8" NI-20	1	1
B13	3-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B15	3-00-00	11 7/8" NI-20	1	1
B16	9-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	1	1
B18 (-3R)	6-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B21 (-3R)	7-00-00	11 7/8" NI-20	1	1
B32	5-00-00	11 7/8" NI-20	1	1
B33	5-00-00	11 7/8" NI-20	1	1
B34	10-00-00	11 7/8" NI-20	1	1
B35	6-00-00	11 7/8" NI-20	1	1
B36	10-00-00	11 7/8" NI-20	2	2
B37 (-3R)	10-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B38 (-3R)	10-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B44	22-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	4	4
J1	13-00-00	11 7/8" NI-20	1	1
J2	10-00-00	11 7/8" NI-20	1	1
J3	6-00-00	11 7/8" NI-20	1	8
J4	4-00-00	11 7/8" NI-20	1	1
J5	3-00-00	11 7/8" NI-20	1	16
J6	22-00-00	11 7/8" NI-40x	2	44
J7	21-00-00	11 7/8" NI-40x	1	24
J8	18-00-00	11 7/8" NI-40x	1	34
Ca1	224-00-00	1 1/8" x 11 7/8" Rim Board	1	1
Bk1	105-00-00	11 7/8" NI-20	1	1

Connector Summary			
PlotID	Qty	Manuf	Product
H1	2		HGUS410
H2	2		HU312-2
H3	46		LT251188

GROUND FLOOR FRAMING
UNIT 5013 - THE RIVERVIEW
ELEVATION C
W/ L.O.D. CONDITION
W/ ELEVATOR

FLOOR LOADING
LIVE LOAD : 40 PSF
DEAD LOAD : 15 PSF
DEAD LOAD (TILE): 20 PSF

HATCH LEGEND
Ceramic Tile
Conv Framed

APP - AS PER PLAN
BBO - BEAM BY OTHERS
PA - POST ABOVE
O.T.B - OPEN TO BELOW
GT - GIRDER TRUSS
RT - ROOF TRUSS

RIMBOARD
1-1/8" X 11-7/8" O.S.B

SUBFLOOR: 3/4" NAILED & GLUED*

Blocking panels are required over all interior supports.
Squash blocks are required under concentrated loads.

Ceramic Tile Application as per O.B.C. 9.30.6

Provide I-Joist blocking between cantilevered joists (along bearing) and rimboard closure at ends.

Do not scale - refer to architectural plans for dimensions.

JT/PL: 45147/116460
LI: 343077*

Builder: Gold Park Homes
Project: Pine Valley Ph2

Location: Vaughan, ON
Date: May. 06, 2022

Designer: TL
Sheet: 12 of 36

Alpa Roof Trusses Inc.
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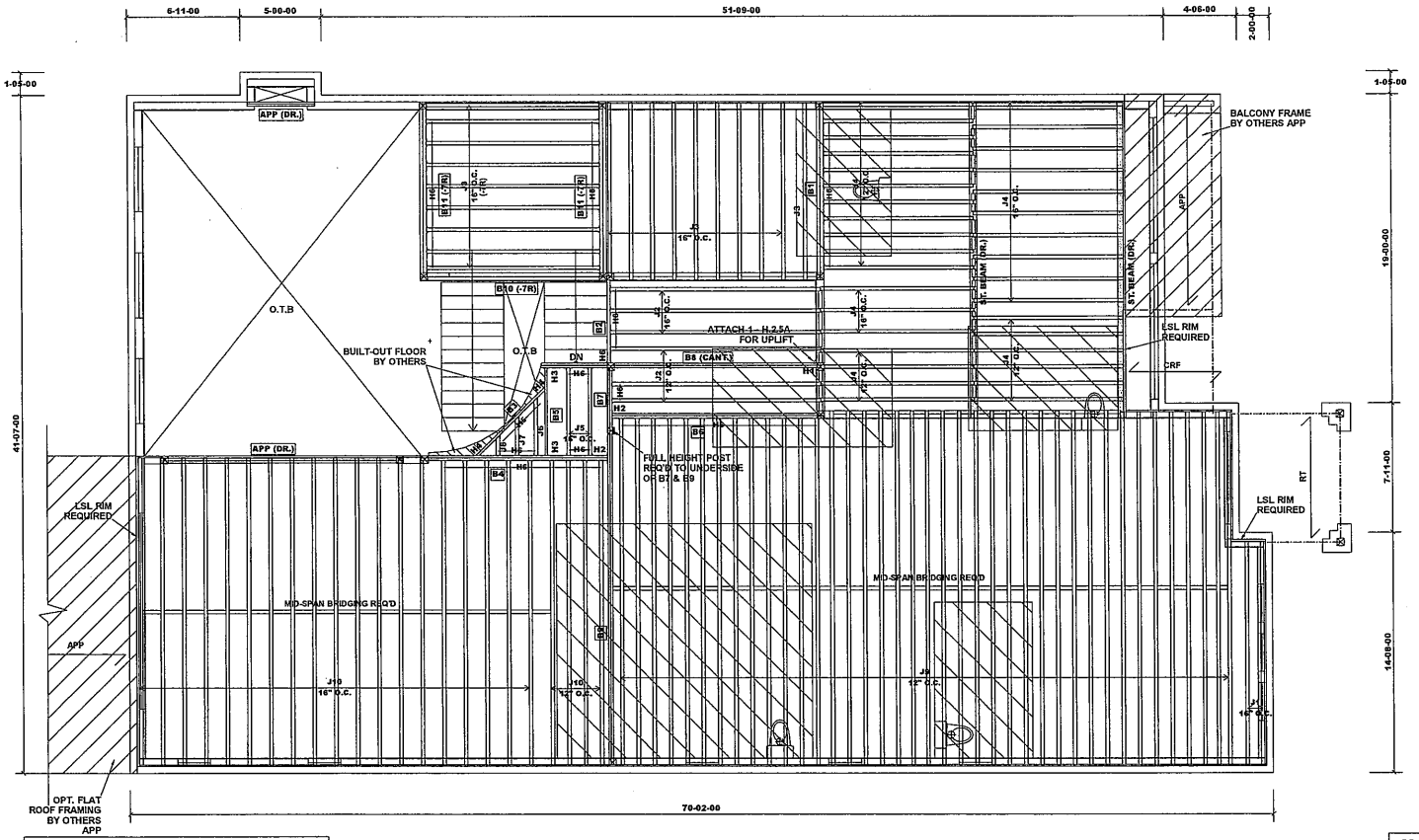

Products				
PlotID	Length	Product	Plies	Net Qty
B1	11-00-00	11 7/8" NI-20	2	2
B2	6-00-00	11 7/8" NI-20	1	1
B3	7-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	1	1
B4	12-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B5	6-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	1	1
B6	14-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B7	4-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B8 (CANT.)	18-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B9	21-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B10 (-7R)	12-00-00	11 7/8" NI-20	1	1
B11 (-7R)	11-00-00	11 7/8" NI-20	2	4
J1	14-00-00	11 7/8" NI-20	1	2
J2	13-00-00	11 7/8" NI-20	1	6
J3	11-00-00	11 7/8" NI-20	1	19
J4	10-00-00	11 7/8" NI-20	1	34
J5	6-00-00	11 7/8" NI-20	1	2
J6	4-00-00	11 7/8" NI-20	1	1
J7	3-00-00	11 7/8" NI-20	1	1
J8	2-00-00	11 7/8" NI-20	1	1
J9	22-00-00	11 7/8" NI-40x	1	39
J10	19-00-00	11 7/8" NI-40x	1	23
Ca1	182-00-00	1 1/8" x 11 7/8" Rim Board	1	1
Ca2	28-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	1	1
Bk1	95-00-00	11 7/8" NI-20	1	1

Connector Summary			
PlotID	Qty	Manuf	Product
H1	1	N/A	H2.5A
H2	2		HGUS410
H3	2		HUS1.81/10
H4	2		LSSR1.81Z
H5	3		LSSR2.56Z
H6	62		LT251188

SECOND FLOOR FRAMING
UNIT 5013 - THE RIVERVIEW
ELEVATION A
W/ LOFT CONDITION

FLOOR LOADING
LIVE LOAD : 40 PSF
DEAD LOAD : 15 PSF
DEAD LOAD (TILE): 20 PSF

HATCH LEGEND
- Ceramic Tile
- Conv Framed

APP - AS PER PLAN
BBO - BEAM BY OTHERS
PA - POST ABOVE
O.T.B - OPEN TO BELOW
GT - GIRDER TRUSS
RT - ROOF TRUSS

RIMBOARD
1-1/8" X 11-7/8" O.S.B

SUBFLOOR: 3/4" NAILED & GLUED*

Blocking panels are required over all interior supports.
Squash blocks are required under concentrated loads.

Ceramic Tile Application as per O.B.C. 9.30.6

Provide I-Joist blocking between cantilevered joists (along bearing) and rimboard closure at ends.

Do not scale - refer to architectural plans for dimensions.

JT/PL: 45147/116460
LI: 343077*

Builder: Gold Park Homes
Project: Pine Valley Ph2

Location: Vaughan, ON
Date: May. 06, 2022

Designer: TL
Sheet: 13 of 36

Alpa Roof Trusses Inc.
Stouffville, Ontario

Salesperson: Derek F.
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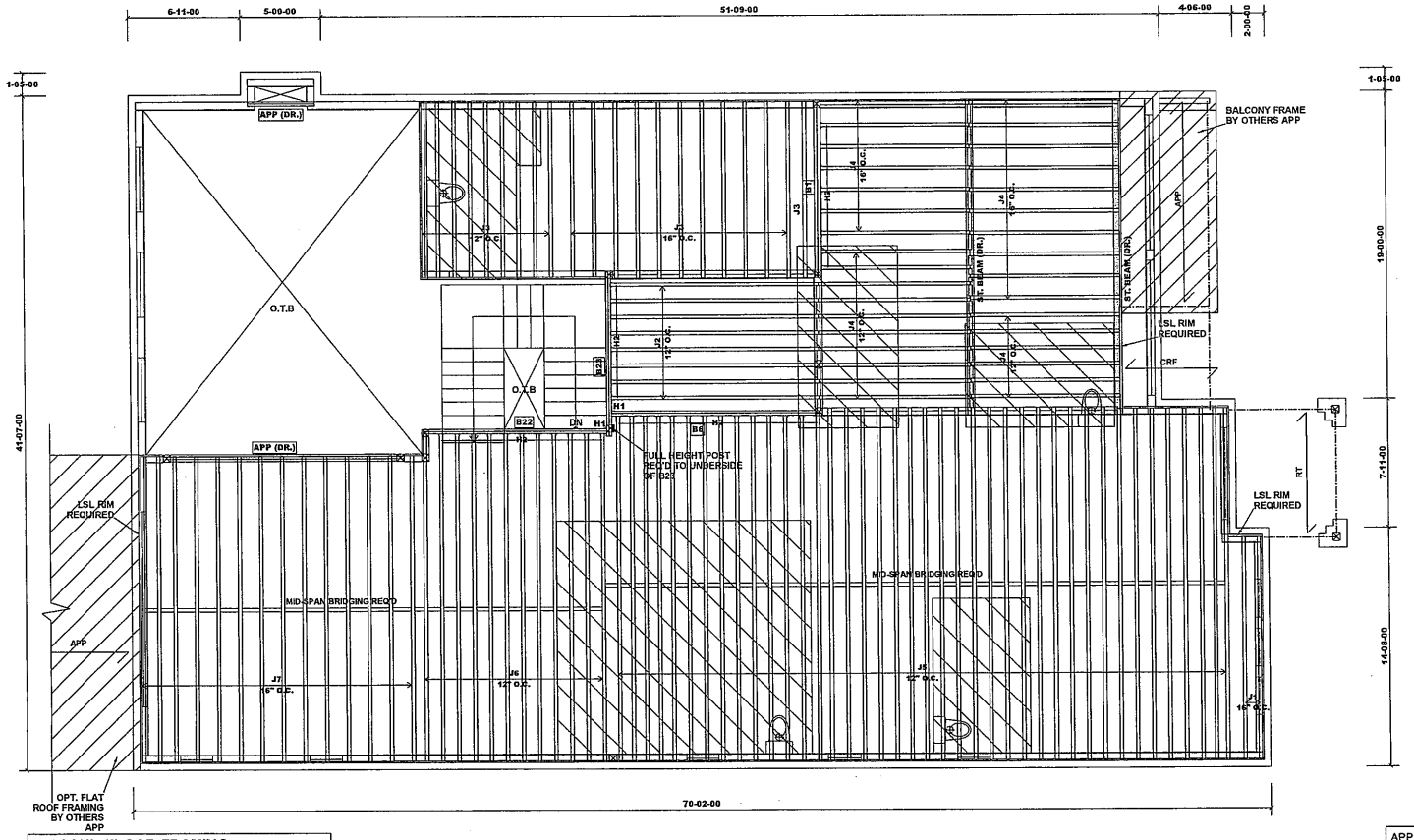


Products				
PlotID	Length	Product	Plies	Net Qty
B1	11-00-00	11 7/8" NI-20	2	2
B6	14-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B22	12-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B23	11-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
J1	14-00-00	11 7/8" NI-20	1	2
J2	13-00-00	11 7/8" NI-20	1	8
J3	11-00-00	11 7/8" NI-20	1	21
J4	10-00-00	11 7/8" NI-20	1	33
J5	22-00-00	11 7/8" NI-40x	1	40
J6	21-00-00	11 7/8" NI-40x	1	12
J7	19-00-00	11 7/8" NI-40x	1	14
Ca1	195-00-00	1 1/8" x 11 7/8" Rim Board	1	1
Ca2	28-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	1	1
Bk1	99-00-00	11 7/8" NI-20	1	1

Connector Summary			
PlotID	Qty	Manuf	Product
H1	2		HGUS410
H2	41		LT251188

SECOND FLOOR FRAMING
UNIT 5013 - THE RIVERVIEW
ELEVATION A
WO/ LOFT CONDITION

FLOOR LOADING
LIVE LOAD : 40 PSF
DEAD LOAD : 15 PSF
DEAD LOAD (TILE): 20 PSF

HATCH LEGEND
- Ceramic Tile
- Conv Framed

APP - AS PER PLAN
BBO - BEAM BY OTHERS
PA - POST ABOVE
O.T.B - OPEN TO BELOW
GT - GIRDER TRUSS
RT - ROOF TRUSS

RIMBOARD
1-1/8" X 11-7/8" O.S.B

SUBFLOOR: 3/4" NAILED & GLUED*

Blocking panels are required over all interior supports.
Squash blocks are required under concentrated loads.

Ceramic Tile Application as per O.B.C. 9.30.6

Provide I-Joist blocking between cantilevered joists (along bearing) and rimboard closure at ends.

Do not scale - refer to architectural plans for dimensions.

JT/PL: 45147/116460
LI: 343077*

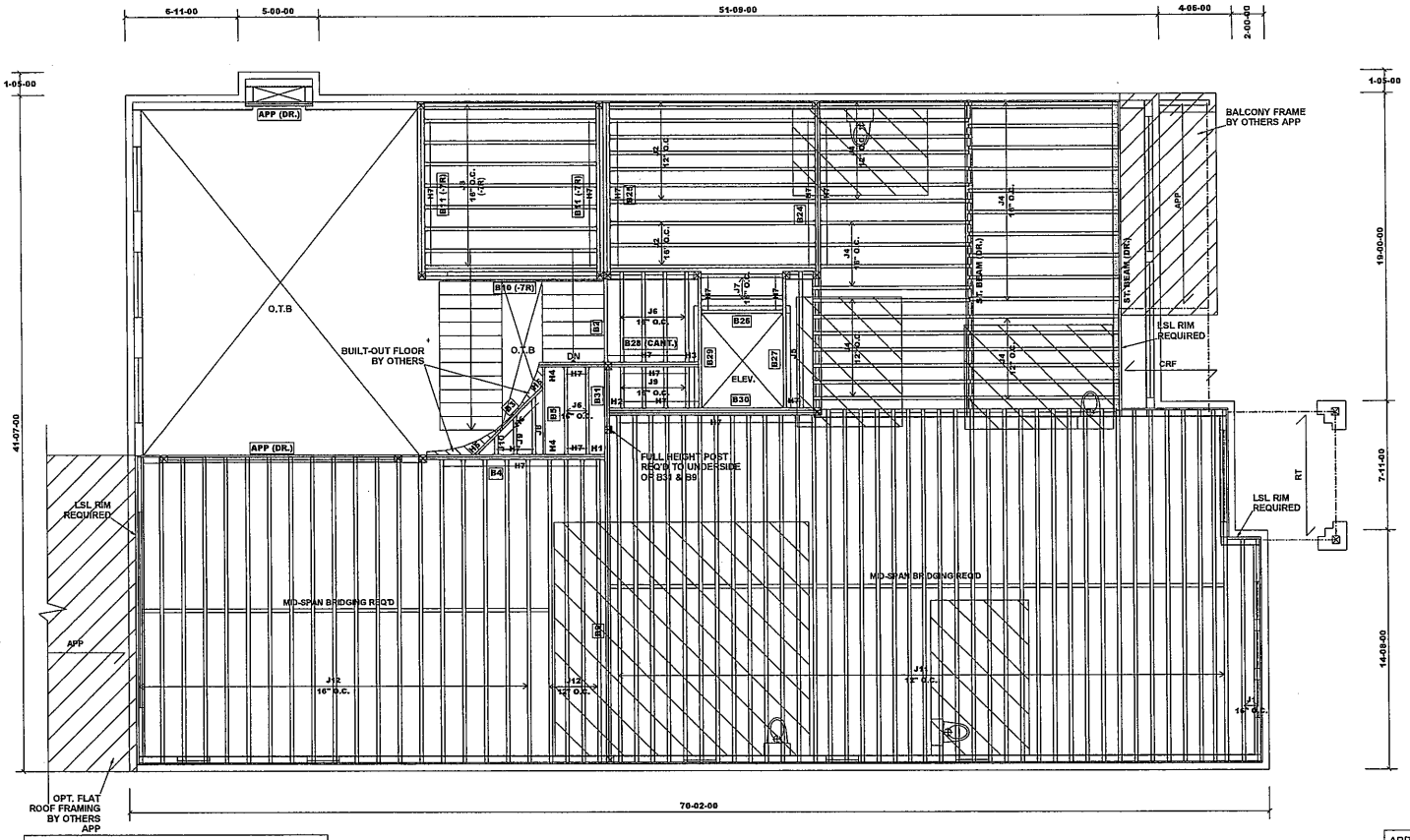
Builder: Gold Park Homes
Project: Pine Valley Ph2

Location: Vaughan, ON
Date: May. 06, 2022

Designer: TL
Sheet: 14 of 36

Alpa Roof Trusses Inc.
Stouffville, Ontario

Salesperson: Derek F.
Home Lumber Inc.



Products				
PlotID	Length	Product	Plies	Net Qty
B2	6-00-00	11 7/8" NI-20	1	1
B3	7-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	1	1
B4	12-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B5	6-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	1	1
B9	21-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B10 (-7R)	12-00-00	11 7/8" NI-20	1	1
B11 (-7R)	11-00-00	11 7/8" NI-20	2	4
B24	11-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B25	11-00-00	11 7/8" NI-20	2	2
B26	5-00-00	11 7/8" NI-20	1	1
B27	9-00-00	11 7/8" NI-20	1	1
B28 (CANT.)	10-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B29	9-00-00	11 7/8" NI-20	1	1
B30	14-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	3	3
B31	4-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
J1	14-00-00	11 7/8" NI-20	1	2
J2	13-00-00	11 7/8" NI-20	1	10
J3	11-00-00	11 7/8" NI-20	1	9
J4	10-00-00	11 7/8" NI-20	1	34
J5	9-00-00	11 7/8" NI-20	1	1
J6	6-00-00	11 7/8" NI-20	1	6
J7	5-00-00	11 7/8" NI-20	1	2
J8	4-00-00	11 7/8" NI-20	1	1
J9	3-00-00	11 7/8" NI-20	1	5
J10	2-00-00	11 7/8" NI-20	1	1
J11	22-00-00	11 7/8" NI-40x	1	39
J12	19-00-00	11 7/8" NI-40x	1	23
Ca1	171-00-00	1 1/8" x 11 7/8" Rim Board	1	1
Ca2	28-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	1	1
Bk1	91-00-00	11 7/8" NI-20	1	1

Connector Summary			
PlotID	Qty	Manuf	Product
H1	1		HGUS410
H2	1		HGUS5.50/10
H3	1		HU410
H4	2		HUS1.81/10
H5	2		LSSR1.81Z
H6	3		LSSR2.56Z
H7	94		LT251188

SECOND FLOOR FRAMING
UNIT 5013 - THE RIVERVIEW
ELEVATION A
W/ LOFT CONDITION
W/ ELEVATOR

FLOOR LOADING
LIVE LOAD : 40 PSF
DEAD LOAD : 15 PSF
DEAD LOAD (TILE): 20 PSF

HATCH LEGEND
Ceramic Tile
Conv Framed

APP - AS PER PLAN
BBO - BEAM BY OTHERS
PA - POST ABOVE
O.T.B - OPEN TO BELOW
GT - GIRDER TRUSS
RT - ROOF TRUSS

RIMBOARD
1-1/8" X 11-7/8" O.S.B

SUBFLOOR: 3/4" NAILED & GLUED*

Blocking panels are required over all interior supports.
Squash blocks are required under concentrated loads.

Ceramic Tile Application as per O.B.C. 9.30.6

Provide I-Joist blocking between cantilevered joists (along bearing) and rimboard closure at ends.

Do not scale - refer to architectural plans for dimensions.

JT/PL: 45147/116460
LI: 343077*

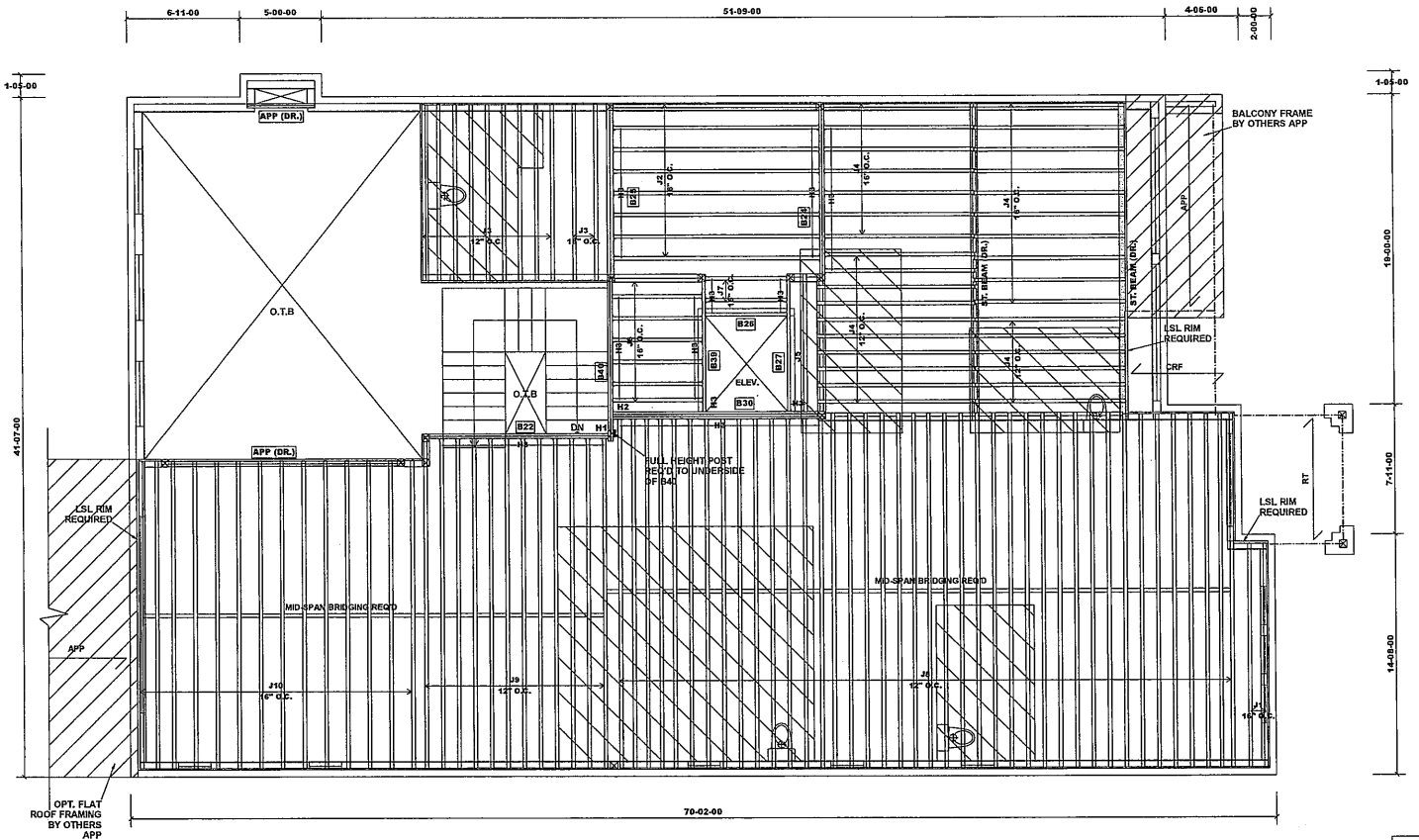
Builder: Gold Park Homes
Project: Pine Valley Ph2

Location: Vaughan, ON
Date: May. 06, 2022

Designer: TL
Sheet: 15 of 36

Alpa Roof Trusses Inc.
Stouffville, Ontario

Salesperson: Derek F.
Home Lumber Inc.



Products				
PlotID	Length	Product	Plies	Net Qty
B22	12-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B24	11-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B25	11-00-00	11 7/8" NI-20	2	2
B26	5-00-00	11 7/8" NI-20	1	1
B27	9-00-00	11 7/8" NI-20	1	1
B30	14-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	3	3
B39	9-00-00	11 7/8" NI-20	1	1
B40	10-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
J1	14-00-00	11 7/8" NI-20	1	2
J2	13-00-00	11 7/8" NI-20	1	8
J3	11-00-00	11 7/8" NI-20	1	11
J4	10-00-00	11 7/8" NI-20	1	33
J5	9-00-00	11 7/8" NI-20	1	1
J6	6-00-00	11 7/8" NI-20	1	7
J7	5-00-00	11 7/8" NI-20	1	2
J8	22-00-00	11 7/8" NI-40x	1	40
J9	21-00-00	11 7/8" NI-40x	1	12
J10	19-00-00	11 7/8" NI-40x	1	14
Ca1	195-00-00	1 1/8" x 11 7/8" Rim Board	1	1
Ca2	26-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	1	1
Blk1	90-00-00	11 7/8" NI-20	1	1

Connector Summary			
PlotID	Qty	Manuf	Product
H1	1		HGUS410
H2	1		HGUS5.50/10
H3	68		LT251188

SECOND FLOOR FRAMING
UNIT 5013 - THE RIVERVIEW
ELEVATION A
WO/ LOFT CONDITION
W/ ELEVATOR

FLOOR LOADING
LIVE LOAD : 40 PSF
DEAD LOAD : 15 PSF
DEAD LOAD (TILE): 20 PSF

HATCH LEGEND
- Ceramic Tile
- Conv Framed

APP - AS PER PLAN
BBO - BEAM BY OTHERS
PA - POST ABOVE
O.T.B - OPEN TO BELOW
GT - GIRDER TRUSS
RT - ROOF TRUSS

RIMBOARD
1-1/8" X 11-7/8" O.S.B

SUBFLOOR: 3/4" NAILED & GLUED*

Blocking panels are required over all interior supports.
Squash blocks are required under concentrated loads.

Ceramic Tile Application as per O.B.C. 9.30.6

Provide I-Joist blocking between cantilevered joists (along bearing) and rimboard closure at ends.

Do not scale - refer to architectural plans for dimensions.

JT/PL: 45147/116460
LI: 343077*

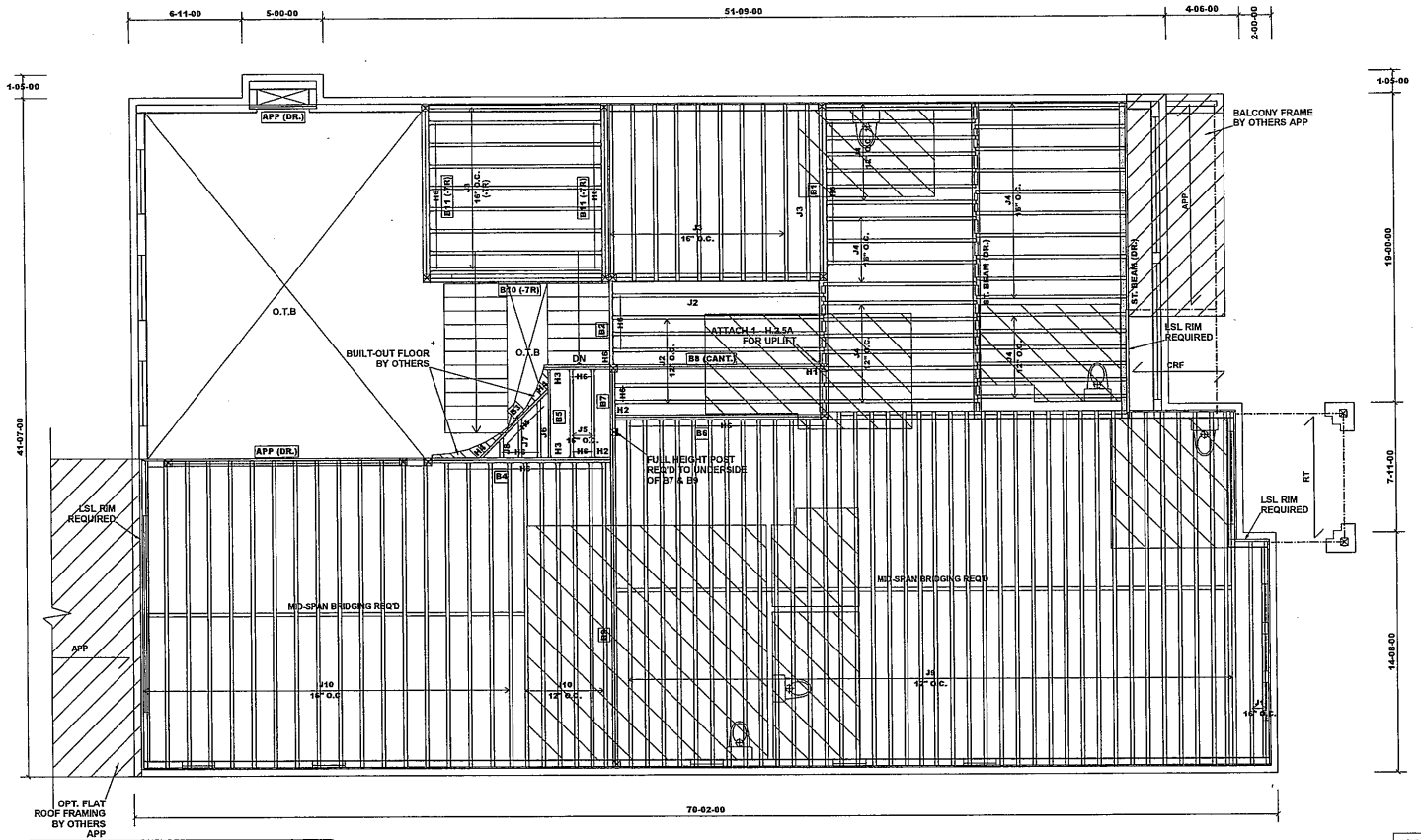
Builder: Gold Park Homes
Project: Pine Valley Ph2

Location: Vaughan, ON
Date: May. 06, 2022

Designer: TL
Sheet: 16 of 36

Alpa Roof Trusses Inc.
Stouffville, Ontario

Salesperson: Derek F.
Home Lumber Inc.



Products				
PlotID	Length	Product	Plies	Net Qty
B1	11-00-00	11 7/8" NI-20	2	2
B2	6-00-00	11 7/8" NI-20	1	1
B3	7-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	1	1
B4	12-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B5	6-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	1	1
B6	14-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B7	4-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B8 (CANT.)	18-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B9	21-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B10 (-7R)	12-00-00	11 7/8" NI-20	1	1
B11 (-7R)	11-00-00	11 7/8" NI-20	2	4
J1	14-00-00	11 7/8" NI-20	1	2
J2	13-00-00	11 7/8" NI-20	1	6
J3	11-00-00	11 7/8" NI-20	1	19
J4	10-00-00	11 7/8" NI-20	1	34
J5	6-00-00	11 7/8" NI-20	1	2
J6	4-00-00	11 7/8" NI-20	1	1
J7	3-00-00	11 7/8" NI-20	1	1
J8	2-00-00	11 7/8" NI-20	1	1
J9	22-00-00	11 7/8" NI-40x	1	39
J10	19-00-00	11 7/8" NI-40x	1	24
Ca1	182-00-00	1 1/8" x 11 7/8" Rim Board	1	1
Ca2	28-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	1	1
Bk1	92-00-00	11 7/8" NI-20	1	1

Connector Summary			
PlotID	Qty	Manuf	Product
H1	1	N/A	H2.5A
H2	2		HGUS410
H3	2		HUS1.81/10
H4	2		LSSR1.81Z
H5	3		LSSR2.56Z
H6	62		LT251188

SECOND FLOOR FRAMING
UNIT 5013 - THE RIVERVIEW
ELEVATION A
W/ LOFT CONDITION
W/ 5 BEDROOM

FLOOR LOADING
LIVE LOAD : 40 PSF
DEAD LOAD : 15 PSF
DEAD LOAD (TILE): 20 PSF

HATCH LEGEND
- Ceramic Tile
- Conv Framed

APP - AS PER PLAN
BBO - BEAM BY OTHERS
PA - POST ABOVE
O.T.B - OPEN TO BELOW
GT - GIRDER TRUSS
RT - ROOF TRUSS

RIMBOARD
1-1/8" X 11-7/8" O.S.B

SUBFLOOR: 3/4" NAILED & GLUED*

Blocking panels are required over all interior supports.
Squash blocks are required under concentrated loads.

Ceramic Tile Application as per O.B.C. 9.30.6

Provide I-Joist blocking between cantilevered joists (along bearing) and rimboard closure at ends.

Do not scale - refer to architectural plans for dimensions.

JT/PL: 45147/116460
LI: 343077*

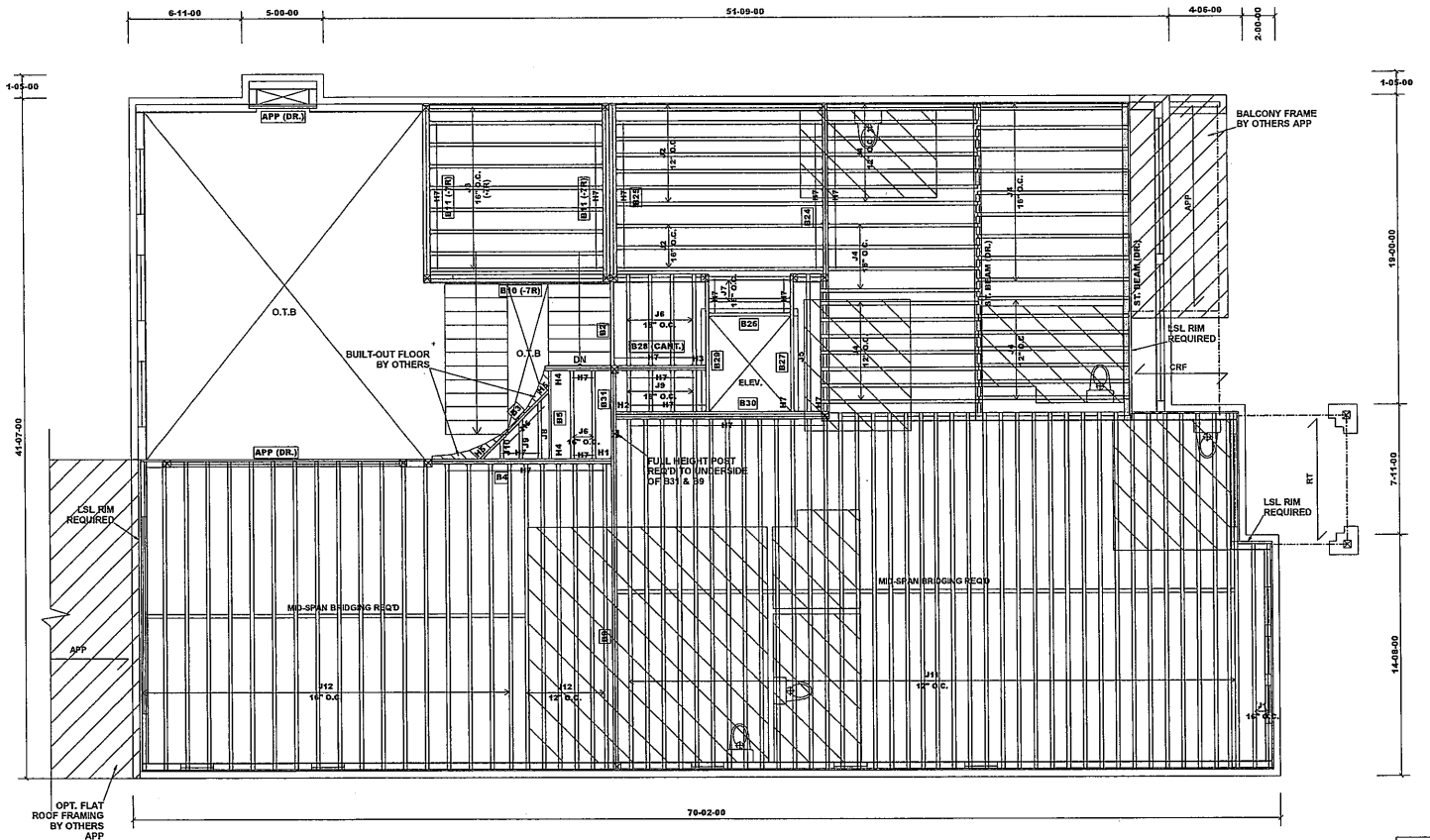
Builder: Gold Park Homes
Project: Pine Valley Ph2

Location: Vaughan, ON
Date: May. 06, 2022

Designer: TL
Sheet: 17 of 36

Alpa Roof Trusses Inc.
Stouffville, Ontario

Salesperson: Derek F.
Home Lumber Inc.



Products				
PlotID	Length	Product	Plies	Net Qty
B2	6-00-00	11 7/8" NI-20	1	1
B3	7-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	1	1
B4	12-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B5	6-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	1	1
B9	21-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B10 (-7R)	12-00-00	11 7/8" NI-20	1	1
B11 (-7R)	11-00-00	11 7/8" NI-20	2	4
B24	11-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B25	11-00-00	11 7/8" NI-20	2	2
B26	5-00-00	11 7/8" NI-20	1	1
B27	9-00-00	11 7/8" NI-20	1	1
B28 (CANT.)	10-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B29	9-00-00	11 7/8" NI-20	1	1
B30	14-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	3	3
B31	4-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
J1	14-00-00	11 7/8" NI-20	1	2
J2	13-00-00	11 7/8" NI-20	1	10
J3	11-00-00	11 7/8" NI-20	1	9
J4	10-00-00	11 7/8" NI-20	1	34
J5	9-00-00	11 7/8" NI-20	1	1
J6	6-00-00	11 7/8" NI-20	1	6
J7	5-00-00	11 7/8" NI-20	1	2
J8	4-00-00	11 7/8" NI-20	1	1
J9	3-00-00	11 7/8" NI-20	1	5
J10	2-00-00	11 7/8" NI-20	1	1
J11	22-00-00	11 7/8" NI-40x	1	39
J12	19-00-00	11 7/8" NI-40x	1	24
Ca1	171-00-00	1 1/8" x 11 7/8" Rim Board	1	1
Ca2	28-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	1	1
Bk1	88-00-00	11 7/8" NI-20	1	1

Connector Summary			
PlotID	Qty	Manuf	Product
H1	1		HGUS410
H2	1		HGUS5.50/10
H3	1		HUJ410
H4	2		HUS1.81/10
H5	2		LSSR1.81Z
H6	3		LSSR2.56Z
H7	95		LT251188

SECOND FLOOR FRAMING
UNIT 5013 - THE RIVERVIEW
ELEVATION A
W/ LOFT CONDITION
W/ ELEVATOR
W/ 5 BEDROOM

FLOOR LOADING
LIVE LOAD : 40 PSF
DEAD LOAD : 15 PSF
DEAD LOAD (TILE): 20 PSF

HATCH LEGEND
Ceramic Tile
Conv Framed

APP - AS PER PLAN
BBO - BEAM BY OTHERS
PA - POST ABOVE
O.T.B - OPEN TO BELOW
GT - GIRDER TRUSS
RT - ROOF TRUSS

RIMBOARD
1-1/8" X 11-7/8" O.S.B

SUBFLOOR: 3/4" NAILED & GLUED*

Blocking panels are required over all interior supports.
Squash blocks are required under concentrated loads.

Ceramic Tile Application as per O.B.C. 9.30.6

Provide I-Joist blocking between cantilevered joists (along bearing) and rimboard closure at ends.

Do not scale - refer to architectural plans for dimensions.

JT/PL: 45147/116460
LI: 343077*

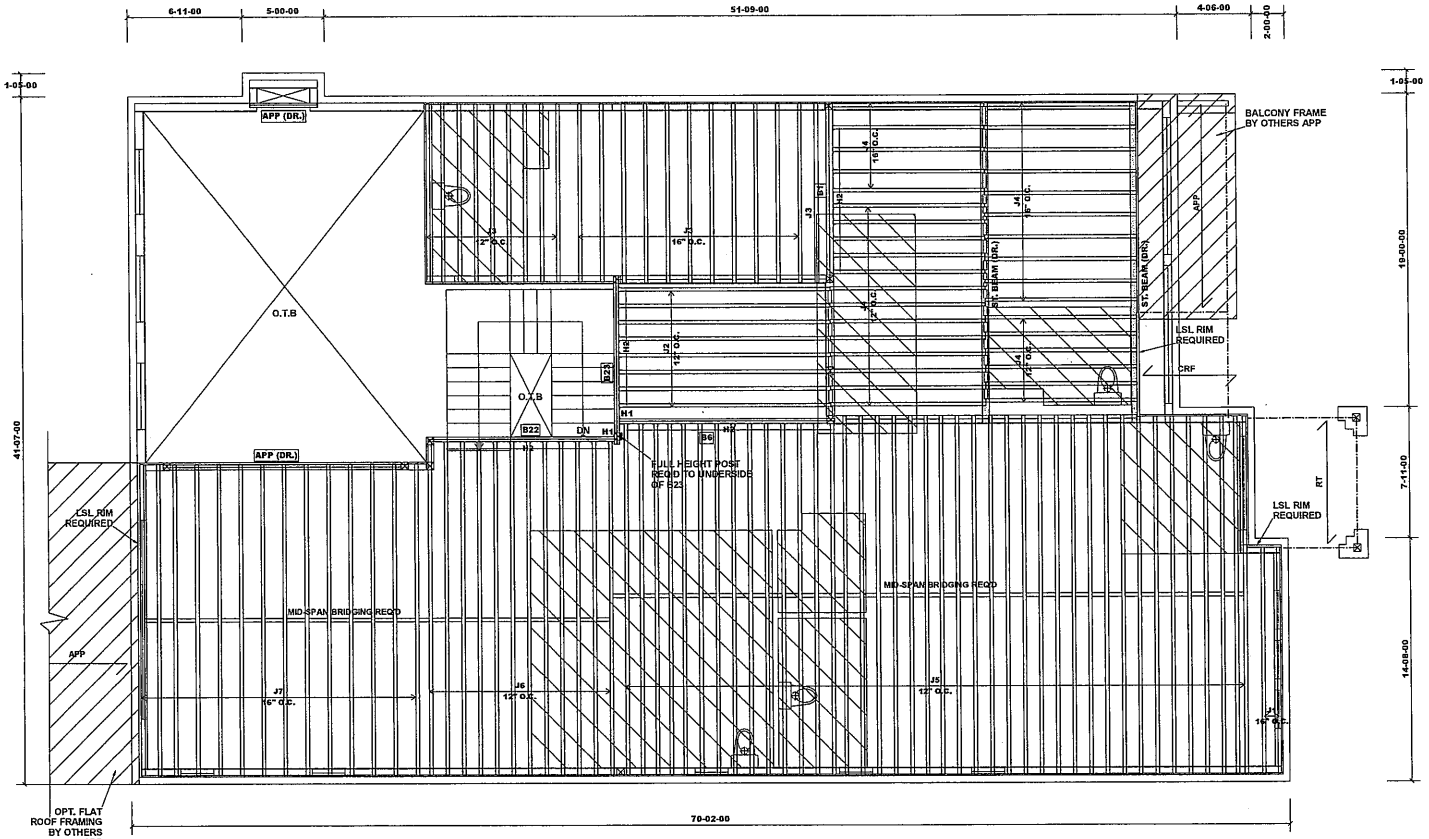
Builder: Gold Park Homes
Project: Pine Valley Ph2

Location: Vaughan, ON
Date: May. 06, 2022

Designer: TL
Sheet: 18 of 36

Alpa Roof Trusses Inc.
Stouffville, Ontario

Salesperson: Derek F.
Home Lumber Inc.



Products				
PlotID	Length	Product	Plies	Net Qty
B1	11-00-00	1 7/8" NI-20	2	2
B8	14-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B22	12-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B23	11-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
J1	14-00-00	1 7/8" NI-20	1	2
J2	13-00-00	1 7/8" NI-20	1	8
J3	11-00-00	1 7/8" NI-20	1	21
J4	10-00-00	1 7/8" NI-20	1	34
J5	22-00-00	1 7/8" NI-40x	1	40
J6	21-00-00	1 7/8" NI-40x	1	12
J7	19-00-00	1 7/8" NI-40x	1	14
Ca1	195-00-00	1 1/8" x 11 7/8" Rim Board	1	1
Ca2	26-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	1	1
Bk1	95-00-00	1 7/8" NI-20	1	1

Connector Summary			
PlotID	Qty	Manuf	Product
H1	2		HGUS410
H2	42		LT251188

SECOND FLOOR FRAMING
UNIT 5013 - THE RIVERVIEW
ELEVATION A
WO/ LOFT CONDITION
W/ 5 BEDROOM

FLOOR LOADING
LIVE LOAD : 40 PSF
DEAD LOAD : 15 PSF
DEAD LOAD (TILE): 20 PSF

HATCH LEGEND
Ceramic Tile
Conv Framed

APP - AS PER PLAN
BBO - BEAM BY OTHERS
PA - POST ABOVE
O.T.B - OPEN TO BELOW
GT - GIRDER TRUSS
RT - ROOF TRUSS

RIMBOARD
1-1/8" X 11-7/8" O.S.B

SUBFLOOR: 3/4" NAILED & GLUED*

Blocking panels are required over all interior supports.
Squash blocks are required under concentrated loads.

Ceramic Tile Application as per O.B.C. 9.30.6

Provide I-Joist blocking between cantilevered joists (along bearing) and rimboard closure at ends.

Do not scale - refer to architectural plans for dimensions.

JT/PL: 45147/116460
LI: 343077*

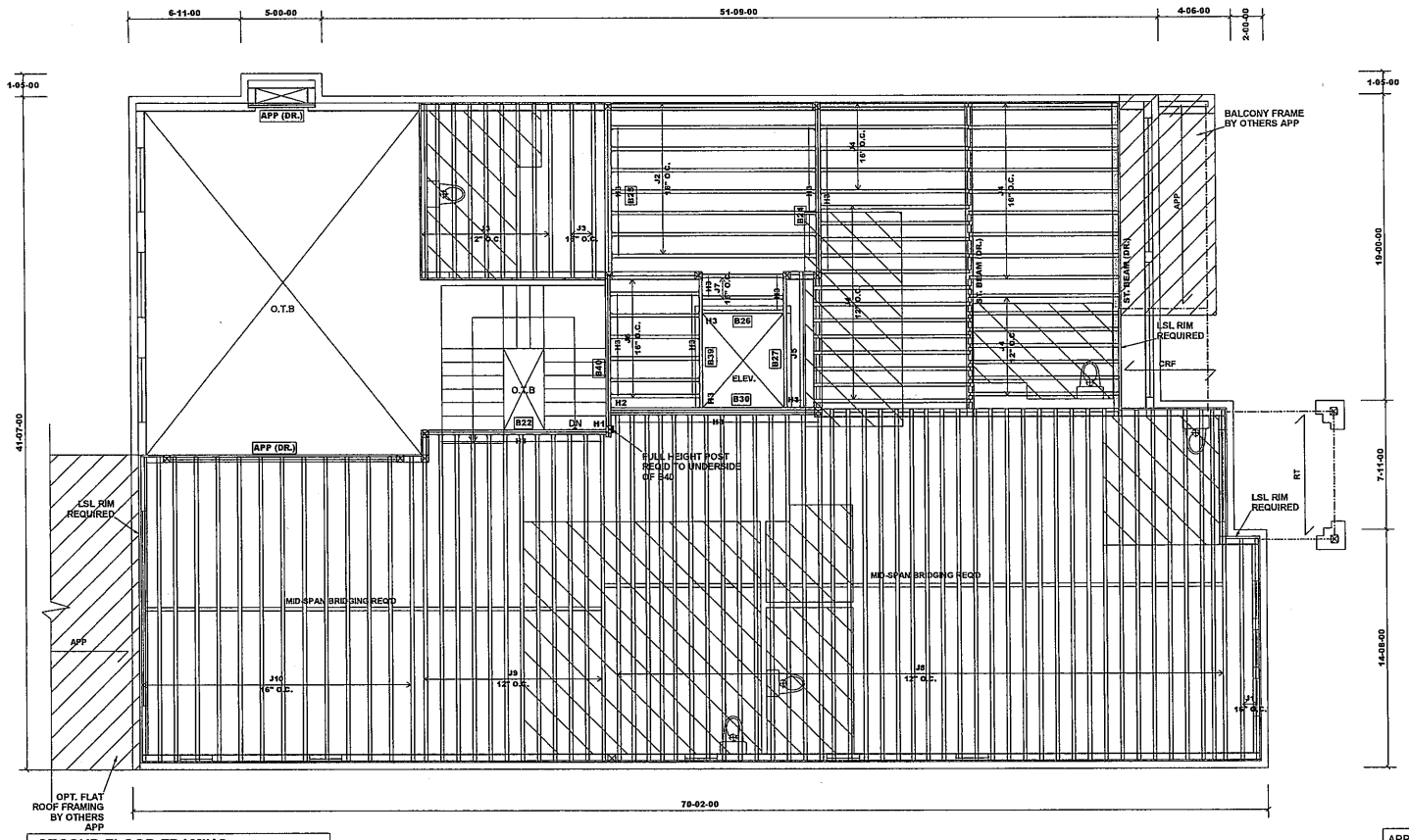
Builder: Gold Park Homes
Project: Pine Valley Ph2

Location: Vaughan, ON
Date: May. 06, 2022

Designer: TL
Sheet: 19 of 36

Alpa Roof Trusses Inc.
Stouffville, Ontario

Salesperson: Derek F.
Home Lumber Inc.



Products				
PlotID	Length	Product	Plies	Net Qty
B22	12-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B24	11-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B25	11-00-00	11 7/8" NI-20	2	2
B26	5-00-00	11 7/8" NI-20	1	1
B27	9-00-00	11 7/8" NI-20	1	1
B30	14-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	3	3
B39	9-00-00	11 7/8" NI-20	1	1
B40	10-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
J1	14-00-00	11 7/8" NI-20	1	2
J2	13-00-00	11 7/8" NI-20	1	8
J3	11-00-00	11 7/8" NI-20	1	11
J4	10-00-00	11 7/8" NI-20	1	34
J5	9-00-00	11 7/8" NI-20	1	1
J6	6-00-00	11 7/8" NI-20	1	7
J7	5-00-00	11 7/8" NI-20	1	2
J8	22-00-00	11 7/8" NI-40x	1	40
J9	21-00-00	11 7/8" NI-40x	1	12
J10	19-00-00	11 7/8" NI-40x	1	14
Ca1	195-00-00	1 1/8" x 11 7/8" Rim Board	1	1
Ca2	28-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	1	1
Bk1	88-00-00	11 7/8" NI-20	1	1

Connector Summary			
PlotID	Qty	Manuf	Product
H1	1		HGUS410
H2	1		HGUS5.50/10
H3	69		LT251188

SECOND FLOOR FRAMING
UNIT 5013 - THE RIVERVIEW
ELEVATION A
WO/ LOFT CONDITION
W/ ELEVATOR
W/ 5 BEDROOM

FLOOR LOADING
LIVE LOAD : 40 PSF
DEAD LOAD : 15 PSF
DEAD LOAD (TILE): 20 PSF

HATCH LEGEND
- Ceramic Tile
- Conv Framed

APP - AS PER PLAN
BBO - BEAM BY OTHERS
PA - POST ABOVE
O.T.B - OPEN TO BELOW
GT - GIRDER TRUSS
RT - ROOF TRUSS

RIMBOARD
1-1/8" X 11-7/8" O.S.B

SUBFLOOR: 3/4" NAILED & GLUED*

Blocking panels are required over all interior supports.
Squash blocks are required under concentrated loads.

Ceramic Tile Application as per O.B.C. 9.30.6

Provide I-Joist blocking between cantilevered joists (along bearing) and rimboard closure at ends.

Do not scale - refer to architectural plans for dimensions.

JT/PL: 45147/116460
LI: 343077*

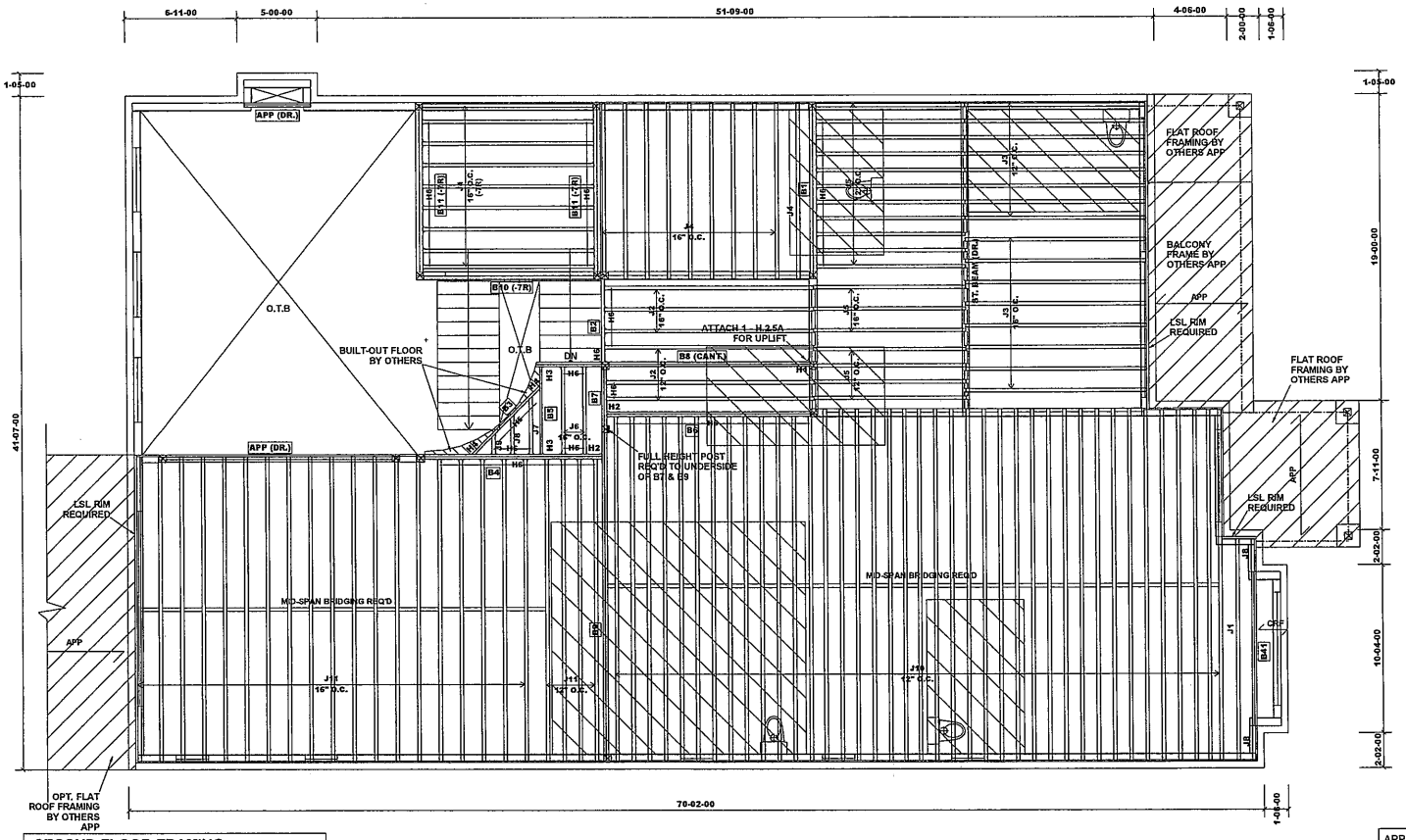
Builder: Gold Park Homes
Project: Pine Valley Ph2

Location: Vaughan, ON
Date: May. 06, 2022

Designer: TL
Sheet: 20 of 36

Alpa Roof Trusses Inc.
Stouffville, Ontario

Salesperson: Derek F.
Home Lumber Inc.



Products				
PlotID	Length	Product	Plies	Net Qty
B1	11-00-00	11 7/8" NI-20	2	2
B2	6-00-00	11 7/8" NI-20	1	1
B3	7-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	1	1
B4	12-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B5	6-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	1	1
B6	14-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B7	4-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B8 (CANT.)	18-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B9	21-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B10 (-7R)	12-00-00	11 7/8" NI-20	1	1
B11 (-7R)	11-00-00	11 7/8" NI-20	2	4
B41	10-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
J1	14-00-00	11 7/8" NI-20	1	1
J2	13-00-00	11 7/8" NI-20	1	6
J3	12-00-00	11 7/8" NI-20	1	16
J4	11-00-00	11 7/8" NI-20	1	19
J5	10-00-00	11 7/8" NI-20	1	18
J6	6-00-00	11 7/8" NI-20	1	2
J7	4-00-00	11 7/8" NI-20	1	1
J8	3-00-00	11 7/8" NI-20	1	3
J9	2-00-00	11 7/8" NI-20	1	1
J10	22-00-00	11 7/8" NI-40x	1	39
J11	19-00-00	11 7/8" NI-40x	1	23
Ca1	159-00-00	1 1/8" x 11 7/8" Rim Board	1	1
Ca2	41-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	1	1
Bk1	97-00-00	11 7/8" NI-20	1	1

Connector Summary			
PlotID	Qty	Manuf	Product
H1	1	N/A	H2.5A
H2	2		HGUS410
H3	2		HUS1.81/10
H4	2		LSSR1.81Z
H5	3		LSSR2.56Z
H6	62		LT251188

SECOND FLOOR FRAMING
UNIT 5013 - THE RIVERVIEW
ELEVATION B
W/ LOFT CONDITION

FLOOR LOADING
LIVE LOAD : 40 PSF
DEAD LOAD : 15 PSF
DEAD LOAD (TILE): 20 PSF

HATCH LEGEND
- Ceramic Tile
- Conv Framed

APP - AS PER PLAN
BBO - BEAM BY OTHERS
PA - POST ABOVE
O.T.B - OPEN TO BELOW
GT - GIRDER TRUSS
RT - ROOF TRUSS

RIMBOARD
1-1/8" X 11-7/8" O.S.B

SUBFLOOR: 3/4" NAILED & GLUED*

Blocking panels are required over all interior supports.
Squash blocks are required under concentrated loads.

Ceramic Tile Application as per O.B.C. 9.30.6

Provide I-Joist blocking between cantilevered joists (along bearing) and rimboard closure at ends.

Do not scale - refer to architectural plans for dimensions.

JT/PL: 45147/116460
LI: 343077*

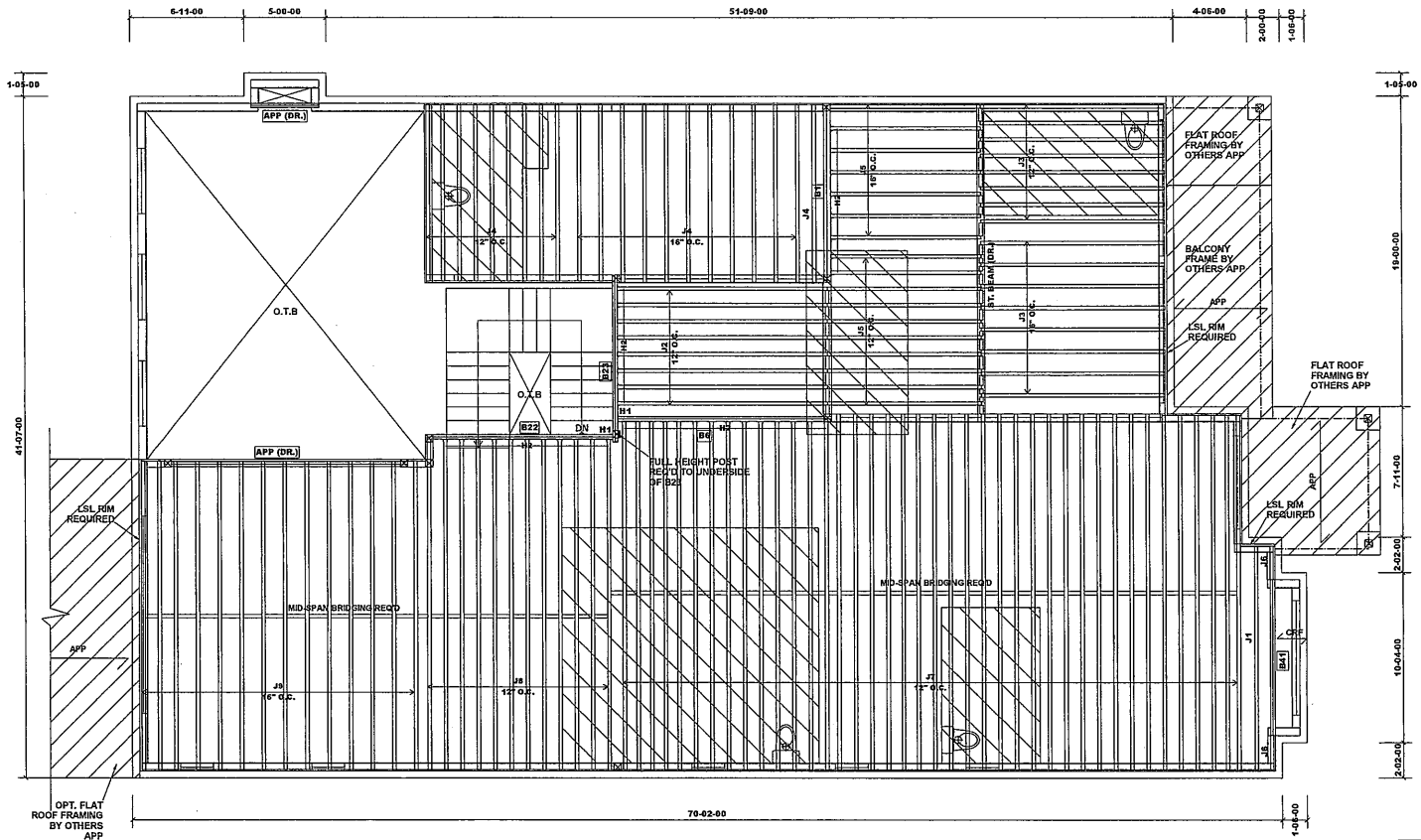
Builder: Gold Park Homes
Project: Pine Valley Ph2

Location: Vaughan, ON
Date: May. 06, 2022

Designer: TL
Sheet: 21 of 36

Alpa Roof Trusses Inc.
Stouffville, Ontario

Salesperson: Derek F.
Home Lumber Inc.



Products				
PlotID	Length	Product	Plies	Net Qty
B1	11-00-00	11 7/8" NI-20	2	2
B6	14-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B22	12-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B23	11-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B41	10-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
J1	14-00-00	11 7/8" NI-20	1	1
J2	13-00-00	11 7/8" NI-20	1	8
J3	12-00-00	11 7/8" NI-20	1	16
J4	11-00-00	11 7/8" NI-20	1	21
J5	10-00-00	11 7/8" NI-20	1	17
J6	3-00-00	11 7/8" NI-20	1	2
J7	22-00-00	11 7/8" NI-40x	1	40
J8	21-00-00	11 7/8" NI-40x	1	12
J9	19-00-00	11 7/8" NI-40x	1	14
Ca1	172-00-00	1 1/8" x 11 7/8" Rim Board	1	1
Ca2	41-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	1	1
Bk1	98-00-00	11 7/8" NI-20	1	1

Connector Summary			
PlotID	Qty	Manuf	Product
H1	2		HGUS410
H2	41		LT251188

SECOND FLOOR FRAMING
UNIT 5013 - THE RIVERVIEW
ELEVATION B
W/O LOFT CONDITION

FLOOR LOADING
LIVE LOAD : 40 PSF
DEAD LOAD : 15 PSF
DEAD LOAD (TILE): 20 PSF

HATCH LEGEND
- Ceramic Tile
- Conv Framed

APP - AS PER PLAN
BBO - BEAM BY OTHERS
PA - POST ABOVE
O.T.B - OPEN TO BELOW
GT - GIRDER TRUSS
RT - ROOF TRUSS

RIMBOARD
1-1/8" X 11-7/8" O.S.B

SUBFLOOR: 3/4" NAILED & GLUED*

Blocking panels are required over all interior supports.
Squash blocks are required under concentrated loads.

Ceramic Tile Application as per O.B.C. 9.30.6

Provide I-Joist blocking between cantilevered joists (along bearing) and rimboard closure at ends.

Do not scale - refer to architectural plans for dimensions.

JT/PL: 45147/116460
LI: 343077*

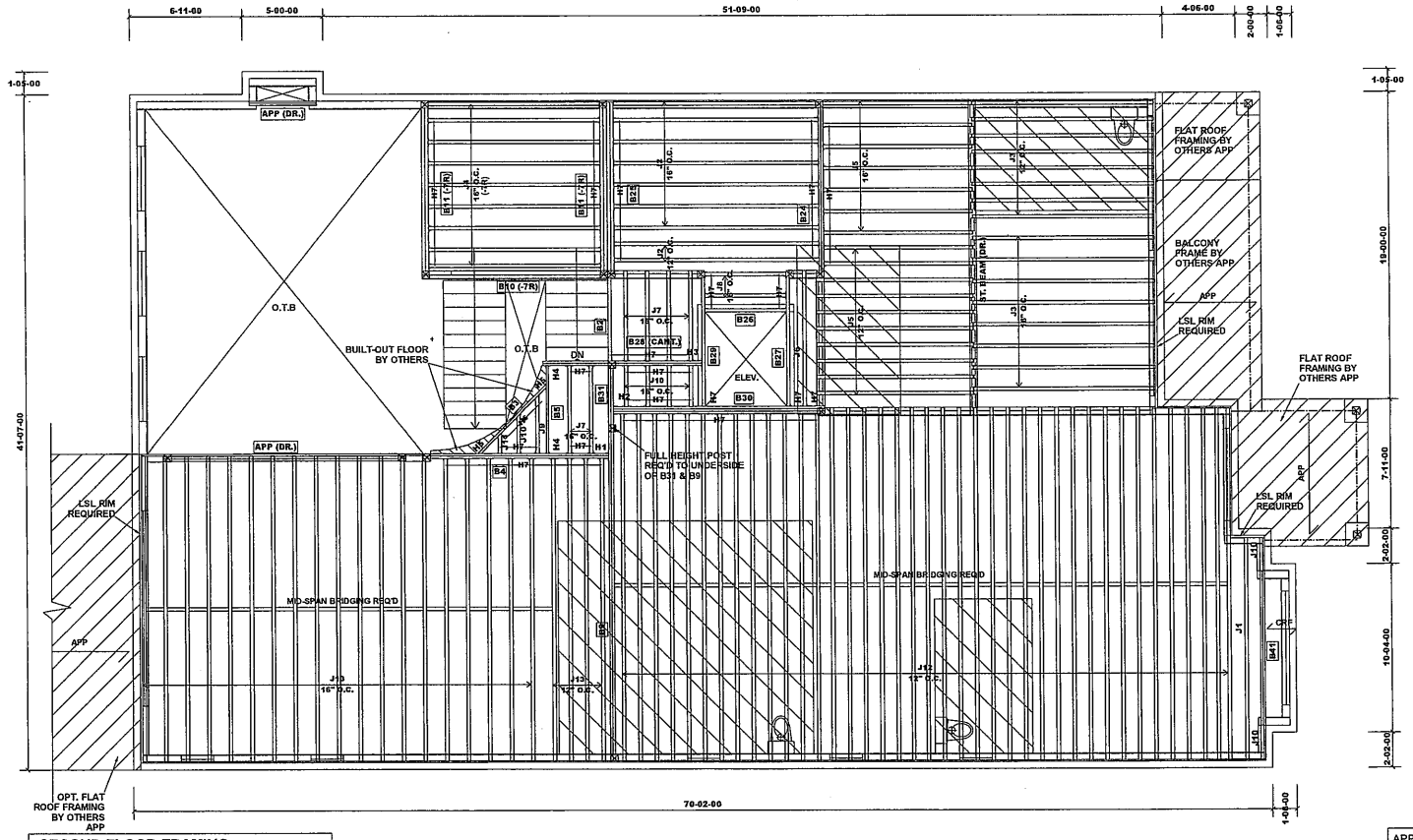
Builder: Gold Park Homes
Project: Pine Valley Ph2

Location: Vaughan, ON
Date: May. 06, 2022

Designer: TL
Sheet: 22 of 36

Alpa Roof Trusses Inc.
Stouffville, Ontario

Salesperson: Derek F.
Home Lumber Inc.



Products				
PlotID	Length	Product	Plies	Net Qty
B2	6-00-00	11 7/8" NI-20	1	1
B3	7-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	1	1
B4	12-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B5	6-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	1	1
B9	21-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B10 (-7R)	12-00-00	11 7/8" NI-20	1	1
B11 (-7R)	11-00-00	11 7/8" NI-20	2	4
B24	11-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B25	11-00-00	11 7/8" NI-20	2	2
B26	5-00-00	11 7/8" NI-20	1	1
B27	9-00-00	11 7/8" NI-20	1	1
B28 (CANT.)	10-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B29	9-00-00	11 7/8" NI-20	1	1
B30	14-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	3	3
B31	4-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B41	10-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
J1	14-00-00	11 7/8" NI-20	1	1
J2	13-00-00	11 7/8" NI-20	1	9
J3	12-00-00	11 7/8" NI-20	1	16
J4	11-00-00	11 7/8" NI-20	1	9
J5	10-00-00	11 7/8" NI-20	1	17
J6	9-00-00	11 7/8" NI-20	1	1
J7	6-00-00	11 7/8" NI-20	1	6
J8	5-00-00	11 7/8" NI-20	1	2
J9	4-00-00	11 7/8" NI-20	1	1
J10	3-00-00	11 7/8" NI-20	1	7
J11	2-00-00	11 7/8" NI-20	1	1
J12	22-00-00	11 7/8" NI-40x	1	39
J13	19-00-00	11 7/8" NI-40x	1	23
Ca1	149-00-00	1 1/8" x 11 7/8" Rim Board	1	1
Ca2	41-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	1	1
Bk1	92-00-00	11 7/8" NI-20	1	1

Connector Summary			
PlotID	Qty	Manuf	Product
H1	1		HGUS410
H2	1		HGUS5.50/10
H3	1		HU410
H4	2		HUS1.81/10
H5	2		LSSR1.81Z
H6	3		LSSR2.56Z
H7	91		LT251188

SECOND FLOOR FRAMING
UNIT 5013 - THE RIVERVIEW
ELEVATION B
W/ LOFT CONDITION
W/ ELEVATOR

FLOOR LOADING
LIVE LOAD : 40 PSF
DEAD LOAD : 15 PSF
DEAD LOAD (TILE): 20 PSF

HATCH LEGEND
- Ceramic Tile
- Conv Framed

APP - AS PER PLAN
BBO - BEAM BY OTHERS
PA - POST ABOVE
O.T.B - OPEN TO BELOW
GT - GIRDER TRUSS
RT - ROOF TRUSS

RIMBOARD
1-1/8" X 11-7/8" O.S.B

SUBFLOOR: 3/4" NAILED & GLUED*

Blocking panels are required over all interior supports.
Squash blocks are required under concentrated loads.

Ceramic Tile Application as per O.B.C. 9.30.6

Provide I-Joist blocking between cantilevered joists (along bearing) and rimboard closure at ends.

Do not scale - refer to architectural plans for dimensions.

JT/PL: 45147/116460
LI: 343077*

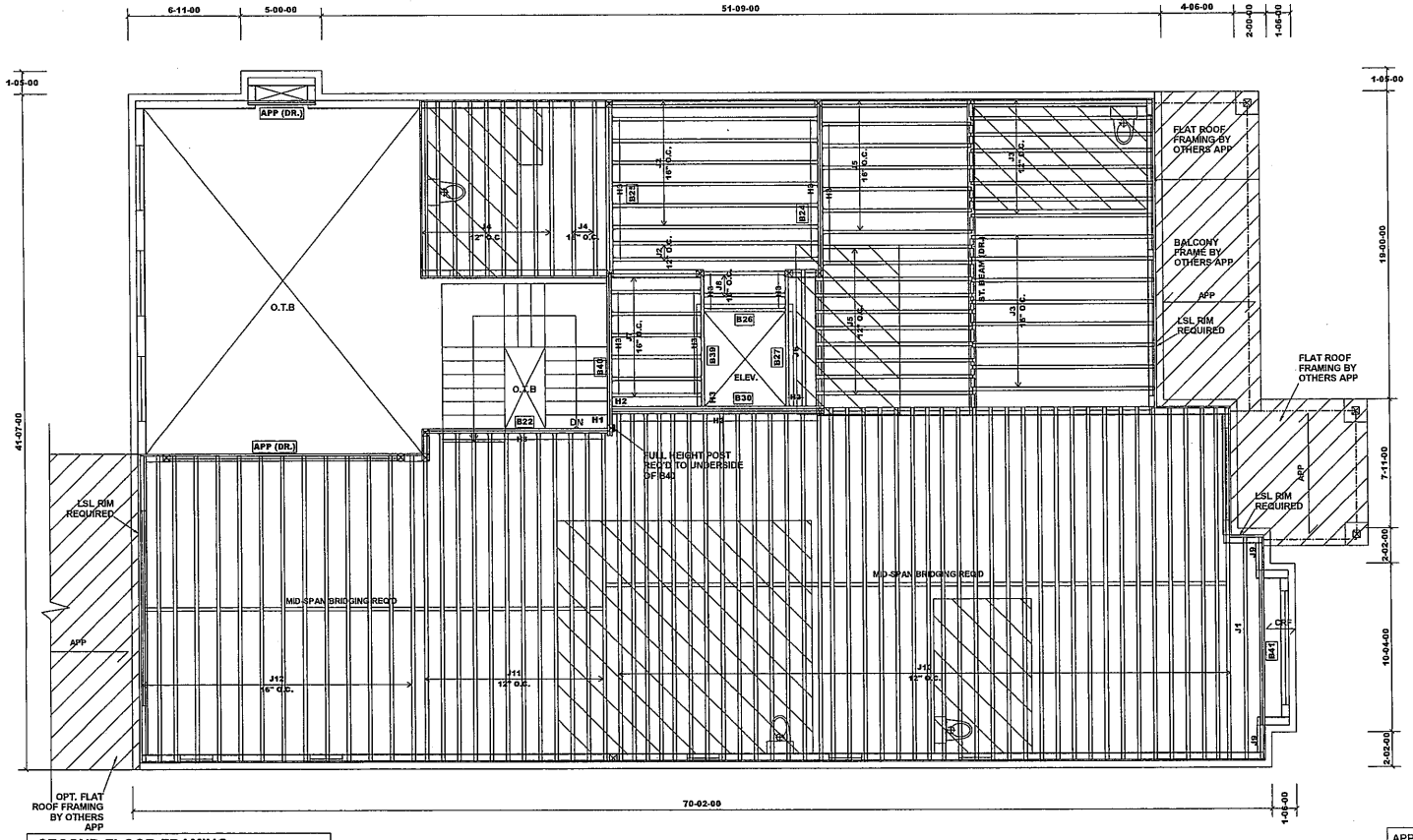
Builder: Gold Park Homes
Project: Pine Valley Ph2

Location: Vaughan, ON
Date: May. 06, 2022

Designer: TL
Sheet: 23 of 36

Alpa Roof Trusses Inc.
Stouffville, Ontario

Salesperson: Derek F.
Home Lumber Inc.



Products				
PlotID	Length	Product	Plies	Net Qty
B22	12-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B24	11-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B25	11-00-00	11 7/8" NI-20	2	2
B26	5-00-00	11 7/8" NI-20	1	1
B27	9-00-00	11 7/8" NI-20	1	1
B30	14-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	3	3
B39	9-00-00	11 7/8" NI-20	1	1
B40	10-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B41	10-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
J1	14-00-00	11 7/8" NI-20	1	1
J2	13-00-00	11 7/8" NI-20	1	9
J3	12-00-00	11 7/8" NI-20	1	16
J4	11-00-00	11 7/8" NI-20	1	11
J5	10-00-00	11 7/8" NI-20	1	17
J6	9-00-00	11 7/8" NI-20	1	1
J7	6-00-00	11 7/8" NI-20	1	7
J8	5-00-00	11 7/8" NI-20	1	2
J9	3-00-00	11 7/8" NI-20	1	2
J10	22-00-00	11 7/8" NI-40x	1	40
J11	21-00-00	11 7/8" NI-40x	1	12
J12	19-00-00	11 7/8" NI-40x	1	14
Ca1	172-00-00	1 1/8" x 11 7/8" Rim Board	1	1
Ca2	41-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	1	1
Bk1	89-00-00	11 7/8" NI-20	1	1

Connector Summary			
PlotID	Qty	Manuf	Product
H1	1		HGUS410
H2	1		HGUS5.50/10
H3	70		LTZ51188

SECOND FLOOR FRAMING
UNIT 5013 - THE RIVERVIEW
ELEVATION B
WO/ LOFT CONDITION
W/ ELEVATOR

FLOOR LOADING
LIVE LOAD : 40 PSF
DEAD LOAD : 15 PSF
DEAD LOAD (TILE): 20 PSF

HATCH LEGEND
- Ceramic Tile
- Conv Framed

APP - AS PER PLAN
BBO - BEAM BY OTHERS
PA - POST ABOVE
O.T.B - OPEN TO BELOW
GT - GIRDER TRUSS
RT - ROOF TRUSS

RIMBOARD
1-1/8" X 11-7/8" O.S.B

SUBFLOOR: 3/4" NAILED & GLUED*

Blocking panels are required over all interior supports.
Squash blocks are required under concentrated loads.

Ceramic Tile Application as per O.B.C. 9.30.6

Provide I-Joist blocking between cantilevered joists (along bearing) and rimboard closure at ends.

Do not scale - refer to architectural plans for dimensions.

JT/PL: 45147/116460
LI: 343077*

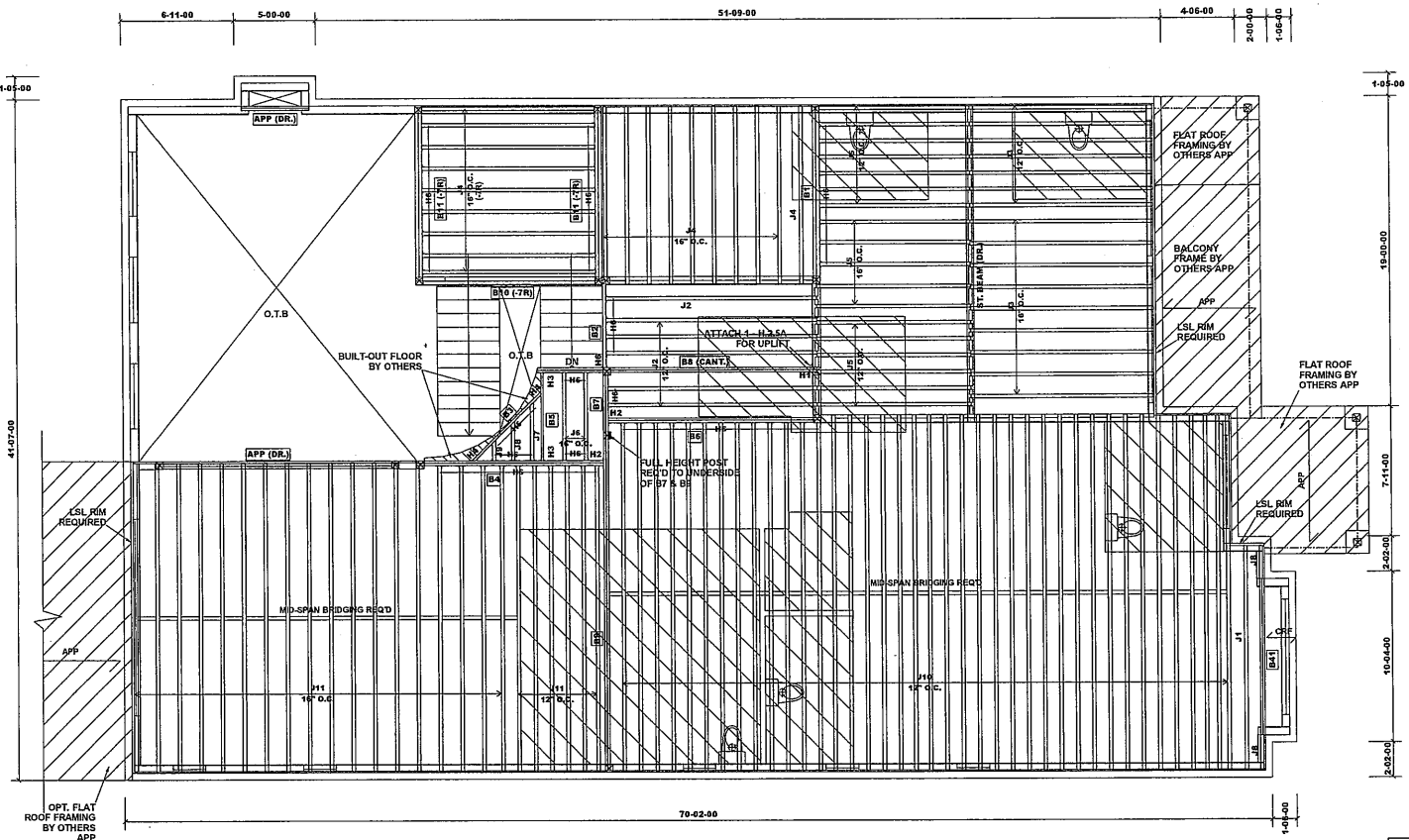
Builder: Gold Park Homes
Project: Pine Valley Ph2

Location: Vaughan, ON
Date: May. 06, 2022

Designer: TL
Sheet: 24 of 36

Alpa Roof Trusses Inc.
Stouffville, Ontario

Salesperson: Derek F.
Home Lumber Inc.



Products				
PlotID	Length	Product	Plies	Net Qty
B1	11-00-00	11 7/8" NI-20	2	2
B2	6-00-00	11 7/8" NI-20	1	1
B3	7-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	1	1
B4	12-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B5	9-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	1	1
B6	14-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B7	4-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B8 (CANT.)	18-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B9	21-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B10 (-7R)	12-00-00	11 7/8" NI-20	1	1
B11 (-7R)	11-00-00	11 7/8" NI-20	2	4
B41	10-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
J1	14-00-00	11 7/8" NI-20	1	1
J2	13-00-00	11 7/8" NI-20	1	6
J3	12-00-00	11 7/8" NI-20	1	16
J4	11-00-00	11 7/8" NI-20	1	19
J5	10-00-00	11 7/8" NI-20	1	18
J6	6-00-00	11 7/8" NI-20	1	2
J7	4-00-00	11 7/8" NI-20	1	1
J8	3-00-00	11 7/8" NI-20	1	3
J9	2-00-00	11 7/8" NI-20	1	1
J10	22-00-00	11 7/8" NI-40x	1	39
J11	19-00-00	11 7/8" NI-40x	1	24
Ca1	159-00-00	1 1/8" x 11 7/8" Rim Board	1	1
Ca2	41-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	1	1
Bk1	96-00-00	11 7/8" NI-20	1	1

Connector Summary			
PlotID	Qty	Manuf	Product
H1	1	N/A	H2.5A
H2	2		HGUS410
H3	2		HUS1.81/10
H4	2		LSSR1.81Z
H5	3		LSSR2.56Z
H6	62		LT251188

SECOND FLOOR FRAMING
UNIT 5013 - THE RIVERVIEW
ELEVATION B
W/ LOFT CONDITION
W/ 5 BEDROOM

FLOOR LOADING
LIVE LOAD : 40 PSF
DEAD LOAD : 15 PSF
DEAD LOAD (TILE): 20 PSF

HATCH LEGEND
- Ceramic Tile
- Conv Framed

APP - AS PER PLAN
BBO - BEAM BY OTHERS
PA - POST ABOVE
O.T.B - OPEN TO BELOW
GT - GIRDER TRUSS
RT - ROOF TRUSS

RIMBOARD
1-1/8" X 11-7/8" O.S.B

SUBFLOOR: 3/4" NAILED & GLUED*

Blocking panels are required over all interior supports.
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Provide I-Joist blocking between cantilevered joists (along bearing) and rimboard closure at ends.

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JT/PL: 45147/116460
LI: 343077*

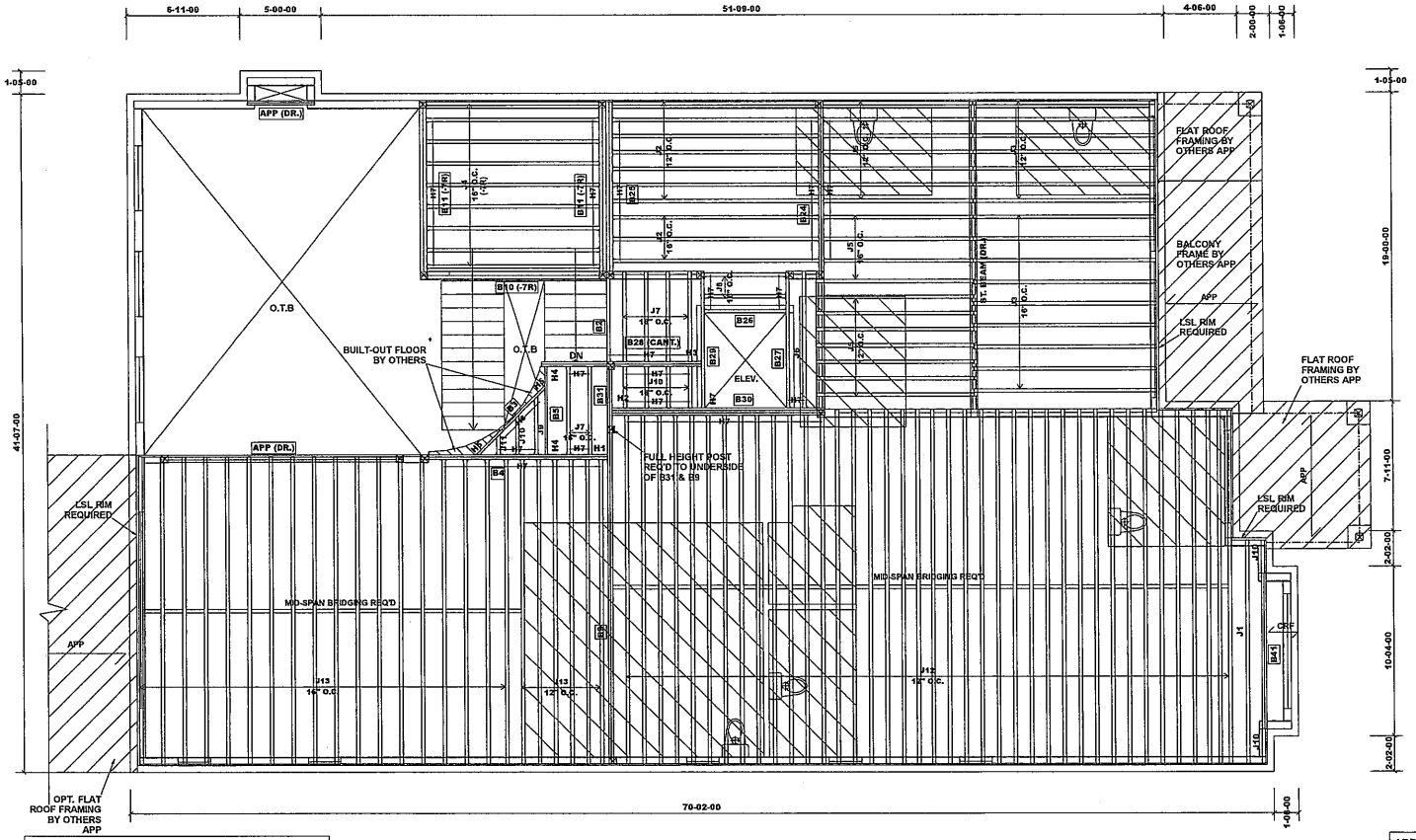
Builder: Gold Park Homes
Project: Pine Valley Ph2

Location: Vaughan, ON
Date: May. 06, 2022

Designer: TL
Sheet: 25 of 36

Alpa Roof Trusses Inc.
Stouffville, Ontario

Salesperson: Derek F.
Home Lumber Inc.



Products				
PieID	Length	Product	Plies	Net Qty
B2	6-00-00	11 7/8" NI-20	1	1
B3	7-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	1	1
B4	12-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B5	6-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	1	1
B9	21-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B10 (-7R)	12-00-00	11 7/8" NI-20	1	1
B11 (-7R)	11-00-00	11 7/8" NI-20	2	4
B24	11-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B25	11-00-00	11 7/8" NI-20	2	2
B26	5-00-00	11 7/8" NI-20	1	1
B27	9-00-00	11 7/8" NI-20	1	1
B28 (CANT.)	10-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B29	9-00-00	11 7/8" NI-20	1	1
B30	14-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	3	3
B31	4-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B41	10-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
J1	14-00-00	11 7/8" NI-20	1	1
J2	13-00-00	11 7/8" NI-20	1	10
J3	12-00-00	11 7/8" NI-20	1	16
J4	11-00-00	11 7/8" NI-20	1	9
J5	10-00-00	11 7/8" NI-20	1	18
J6	9-00-00	11 7/8" NI-20	1	1
J7	6-00-00	11 7/8" NI-20	1	6
J8	5-00-00	11 7/8" NI-20	1	2
J9	4-00-00	11 7/8" NI-20	1	1
J10	3-00-00	11 7/8" NI-20	1	7
J11	2-00-00	11 7/8" NI-20	1	1
J12	22-00-00	11 7/8" NI-40x	1	39
J13	19-00-00	11 7/8" NI-40x	1	24
Ca1	149-00-00	1 1/8" x 11 7/8" Rim Board	1	1
Ca2	41-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	1	1
Bk1	92-00-00	11 7/8" NI-20	1	1

Connector Summary			
PieID	Qty	Manuf	Product
H1	1		HGUS410
H2	1		HGUS5.50/10
H3	1		HU410
H4	2		HUS1.81/10
H5	2		LSSR1.81Z
H6	3		LSSR2.56Z
H7	95		LT251188

SECOND FLOOR FRAMING
UNIT 5013 - THE RIVERVIEW
ELEVATION B
W/ LOFT CONDITION
W/ ELEVATOR
W/ 5 BEDROOM

FLOOR LOADING
LIVE LOAD : 40 PSF
DEAD LOAD : 15 PSF
DEAD LOAD (TILE): 20 PSF

HATCH LEGEND
- Ceramic Tile
- Conv Framed

APP - AS PER PLAN
BBO - BEAM BY OTHERS
PA - POST ABOVE
O.T.B - OPEN TO BELOW
GT - GIRDER TRUSS
RT - ROOF TRUSS

RIMBOARD
1-1/8" X 11-7/8" O.S.B

SUBFLOOR: 3/4" NAILED & GLUED*

Blocking panels are required over all interior supports.
Squash blocks are required under concentrated loads.

Ceramic Tile Application as per O.B.C. 9.30.6

Provide I-Joist blocking between cantilevered joists (along bearing) and rimboard closure at ends.

Do not scale - refer to architectural plans for dimensions.

JT/PL: 45147/116460
LI: 343077*

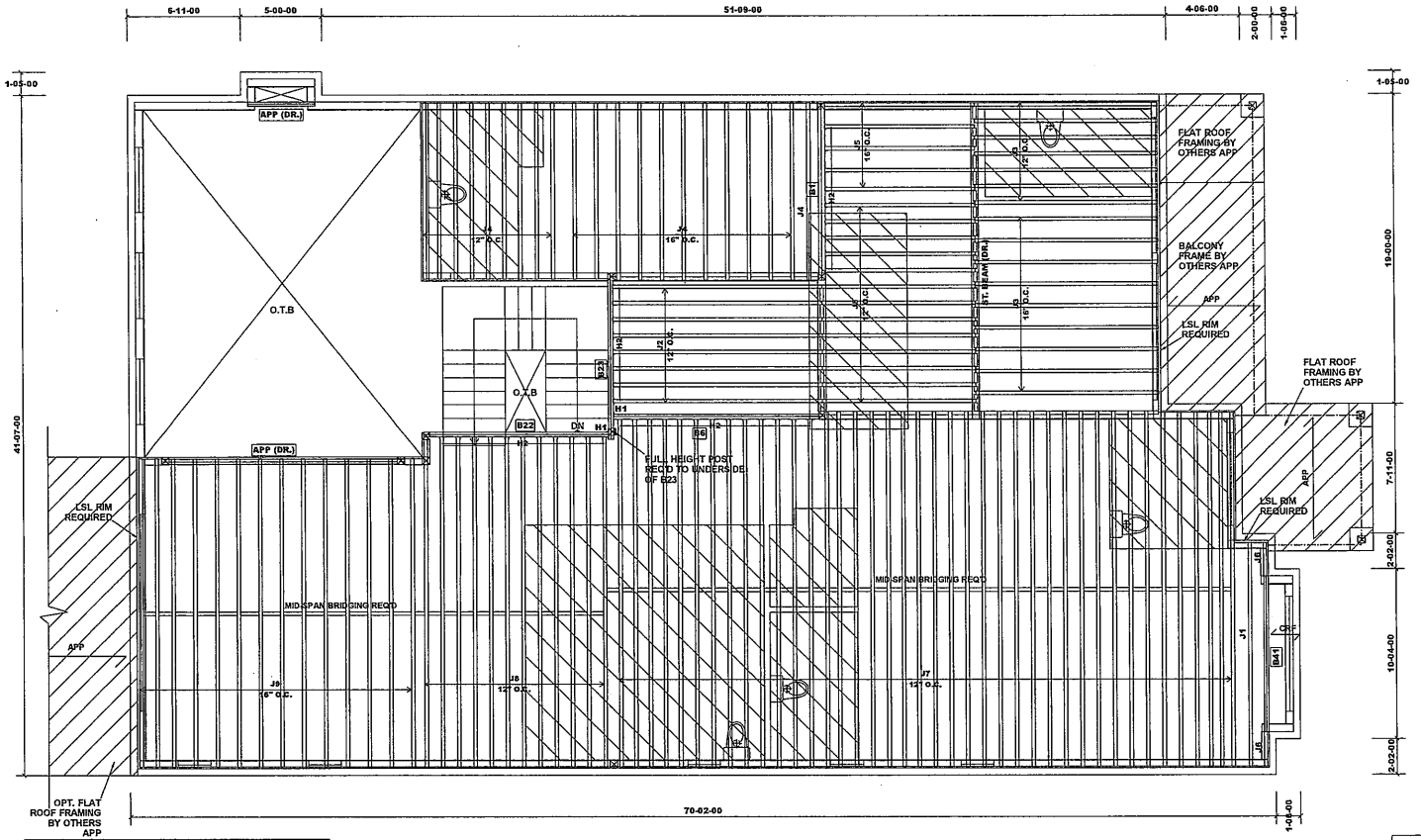
Builder: Gold Park Homes
Project: Pine Valley Ph2

Location: Vaughan, ON
Date: May. 06, 2022

Designer: TL
Sheet: 26 of 36

Alpa Roof Trusses Inc.
Stouffville, Ontario

Salesperson: Derek F.
Home Lumber Inc.



Products				
PlotID	Length	Product	Plies	Net Qty
B1	11-00-00	11 7/8" NI-20	2	2
B6	13-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B22	12-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B23	11-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B41	10-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
J1	14-00-00	11 7/8" NI-20	1	1
J2	13-00-00	11 7/8" NI-20	1	8
J3	12-00-00	11 7/8" NI-20	1	16
J4	11-00-00	11 7/8" NI-20	1	21
J5	10-00-00	11 7/8" NI-20	1	18
J6	3-00-00	11 7/8" NI-20	1	2
J7	22-00-00	11 7/8" NI-40x	1	40
J8	21-00-00	11 7/8" NI-40x	1	12
J9	19-00-00	11 7/8" NI-40x	1	14
Ca1	172-00-00	1 1/8" x 11 7/8" Rim Board	1	1
Ca2	41-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	1	1
Bk1	99-00-00	11 7/8" NI-20	1	1

Connector Summary			
PlotID	Qty	Manuf	Product
H1	2		HGUS410
H2	41		LT251188

SECOND FLOOR FRAMING
UNIT 5013 - THE RIVERVIEW
ELEVATION B
WO/ LOFT CONDITION
W/ 5 BEDROOM

FLOOR LOADING
LIVE LOAD : 40 PSF
DEAD LOAD : 15 PSF
DEAD LOAD (TILE): 20 PSF

HATCH LEGEND
Ceramic Tile
Conv Framed

APP - AS PER PLAN
BBO - BEAM BY OTHERS
PA - POST ABOVE
O.T.B - OPEN TO BELOW
GT - GIRDER TRUSS
RT - ROOF TRUSS

RIMBOARD
1-1/8" X 11-7/8" O.S.B

SUBFLOOR: 3/4" NAILED & GLUED*

Blocking panels are required over all interior supports.
Squash blocks are required under concentrated loads.

Ceramic Tile Application as per O.B.C. 9.30.6

Provide I-Joist blocking between cantilevered joists (along bearing) and rimboard closure at ends.

Do not scale - refer to architectural plans for dimensions.

JT/PL: 45147/116460
LI: 343077*

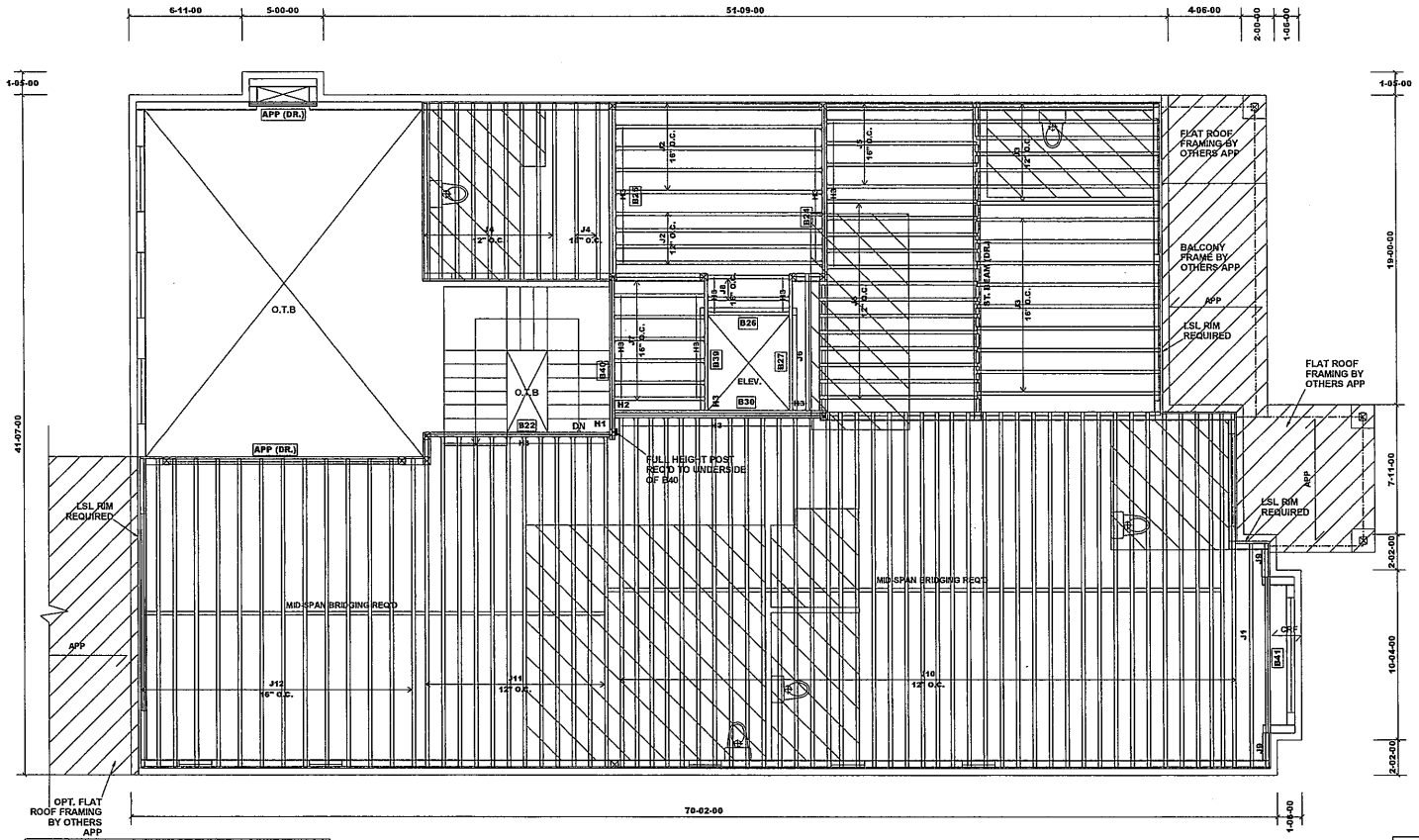
Builder: Gold Park Homes
Project: Pine Valley Ph2

Location: Vaughan, ON
Date: May. 06, 2022

Designer: TL
Sheet: 27 of 36

Alpa Roof Trusses Inc.
Stouffville, Ontario

Salesperson: Derek F.
Home Lumber Inc.



Products				
PlotID	Length	Product	Plies	Net Qty
B22	12-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B24	11-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B25	11-00-00	11 7/8" NI-20	2	2
B26	5-00-00	11 7/8" NI-20	1	1
B27	9-00-00	11 7/8" NI-20	1	1
B30	13-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	3	3
B39	9-00-00	11 7/8" NI-20	1	1
B40	10-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B41	10-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
J1	14-00-00	11 7/8" NI-20	1	1
J2	13-00-00	11 7/8" NI-20	1	9
J3	12-00-00	11 7/8" NI-20	1	16
J4	11-00-00	11 7/8" NI-20	1	11
J5	10-00-00	11 7/8" NI-20	1	18
J6	9-00-00	11 7/8" NI-20	1	1
J7	6-00-00	11 7/8" NI-20	1	7
J8	5-00-00	11 7/8" NI-20	1	2
J9	3-00-00	11 7/8" NI-20	1	2
J10	22-00-00	11 7/8" NI-40x	1	40
J11	21-00-00	11 7/8" NI-40x	1	12
J12	19-00-00	11 7/8" NI-40x	1	14
Ca1	172-00-00	1 1/8" x 11 7/8" Rim Board	1	1
Ca2	41-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	1	1
Blk1	90-00-00	11 7/8" NI-20	1	1

Connector Summary			
PlotID	Qty	Manuf	Product
H1	1		HGUS410
H2	1		HGUS5.50/10
H3	70		LT251188

SECOND FLOOR FRAMING
UNIT 5013 - THE RIVERVIEW
ELEVATION B
WO/ LOFT CONDITION
W/ ELEVATOR
W/ 5 BEDROOM

FLOOR LOADING
LIVE LOAD : 40 PSF
DEAD LOAD : 15 PSF
DEAD LOAD (TILE): 20 PSF

HATCH LEGEND
Ceramic Tile
Conv Framed

APP - AS PER PLAN
BBO - BEAM BY OTHERS
PA - POST ABOVE
O.T.B - OPEN TO BELOW
GT - GIRDER TRUSS
RT - ROOF TRUSS

RIMBOARD
1-1/8" X 11-7/8" O.S.B

SUBFLOOR: 3/4" NAILED & GLUED*

Blocking panels are required over all interior supports.
Squash blocks are required under concentrated loads.

Ceramic Tile Application as per O.B.C. 9.30.6

Provide I-Joist blocking between cantilevered joists (along bearing) and rimboard closure at ends.

Do not scale - refer to architectural plans for dimensions.

JT/PL: 45147/116460
LI: 343077*

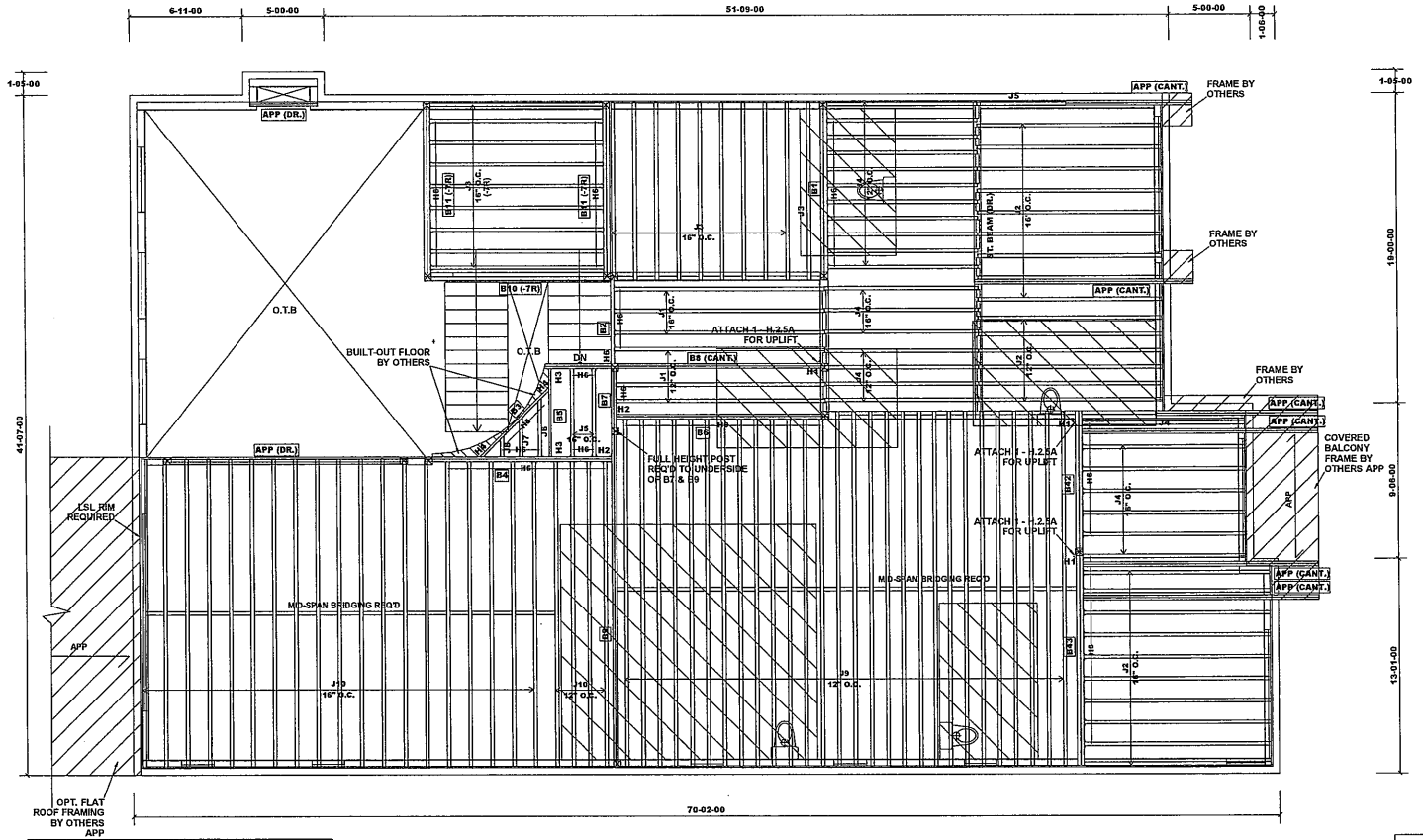
Builder: Gold Park Homes
Project: Pine Valley Ph2

Location: Vaughan, ON
Date: May. 06, 2022

Designer: TL
Sheet: 28 of 36

Alpa Roof Trusses Inc.
Stouffville, Ontario

Salesperson: Derek F.
Home Lumber Inc.



Products				
PlotID	Length	Product	Plies	Net Qty
B1	11-00-00	11 7/8" NI-20	2	2
B2	6-00-00	11 7/8" NI-20	1	1
B3	7-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	1	1
B4	12-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B5	6-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	1	1
B6	14-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B7	4-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B8 (CANT.)	18-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B9	21-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B10 (-7R)	12-00-00	11 7/8" NI-20	1	1
B11 (-7R)	11-00-00	11 7/8" NI-20	2	4
B42	9-00-00	11 7/8" NI-20	2	2
B43	14-00-00	11 7/8" NI-20	2	2
J1	13-00-00	11 7/8" NI-20	1	6
J2	12-00-00	11 7/8" NI-20	1	25
J3	11-00-00	11 7/8" NI-20	1	19
J4	10-00-00	11 7/8" NI-20	1	25
J5	8-00-00	11 7/8" NI-20	1	3
J6	4-00-00	11 7/8" NI-20	1	1
J7	3-00-00	11 7/8" NI-20	1	1
J8	2-00-00	11 7/8" NI-20	1	1
J9	22-00-00	11 7/8" NI-40x	1	28
J10	19-00-00	11 7/8" NI-40x	1	23
Ca1	176-00-00	1 1/8" x 11 7/8" Rim Board	1	1
Ca2	19-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	1	1
Bk1	89-00-00	11 7/8" NI-20	1	1

Connector Summary			
PlotID	Qty	Manuf	Product
H1	3	N/A	H2.5A
H2	2		HGUS410
H3	2		HUS1.81/10
H4	2		LSSR1.81Z
H5	3		LSSR2.56Z
H6	78		LT251188

SECOND FLOOR FRAMING
UNIT 5013 - THE RIVERVIEW
ELEVATION C
W/ LOFT CONDITION

FLOOR LOADING
LIVE LOAD : 40 PSF
DEAD LOAD : 15 PSF
DEAD LOAD (TILE): 20 PSF

HATCH LEGEND
- Ceramic Tile
- Conv Framed

APP - AS PER PLAN
BBO - BEAM BY OTHERS
PA - POST ABOVE
O.T.B - OPEN TO BELOW
GT - GIRDER TRUSS
RT - ROOF TRUSS

RIMBOARD
1-1/8" X 11-7/8" O.S.B

SUBFLOOR: 3/4" NAILED & GLUED*

Blocking panels are required over all interior supports.
Squash blocks are required under concentrated loads.

Ceramic Tile Application as per O.B.C. 9.30.6

Provide I-Joist blocking between cantilevered joists (along bearing) and rimboard closure at ends.

Do not scale - refer to architectural plans for dimensions.

JT/PL: 45147/116460
LI: 343077*

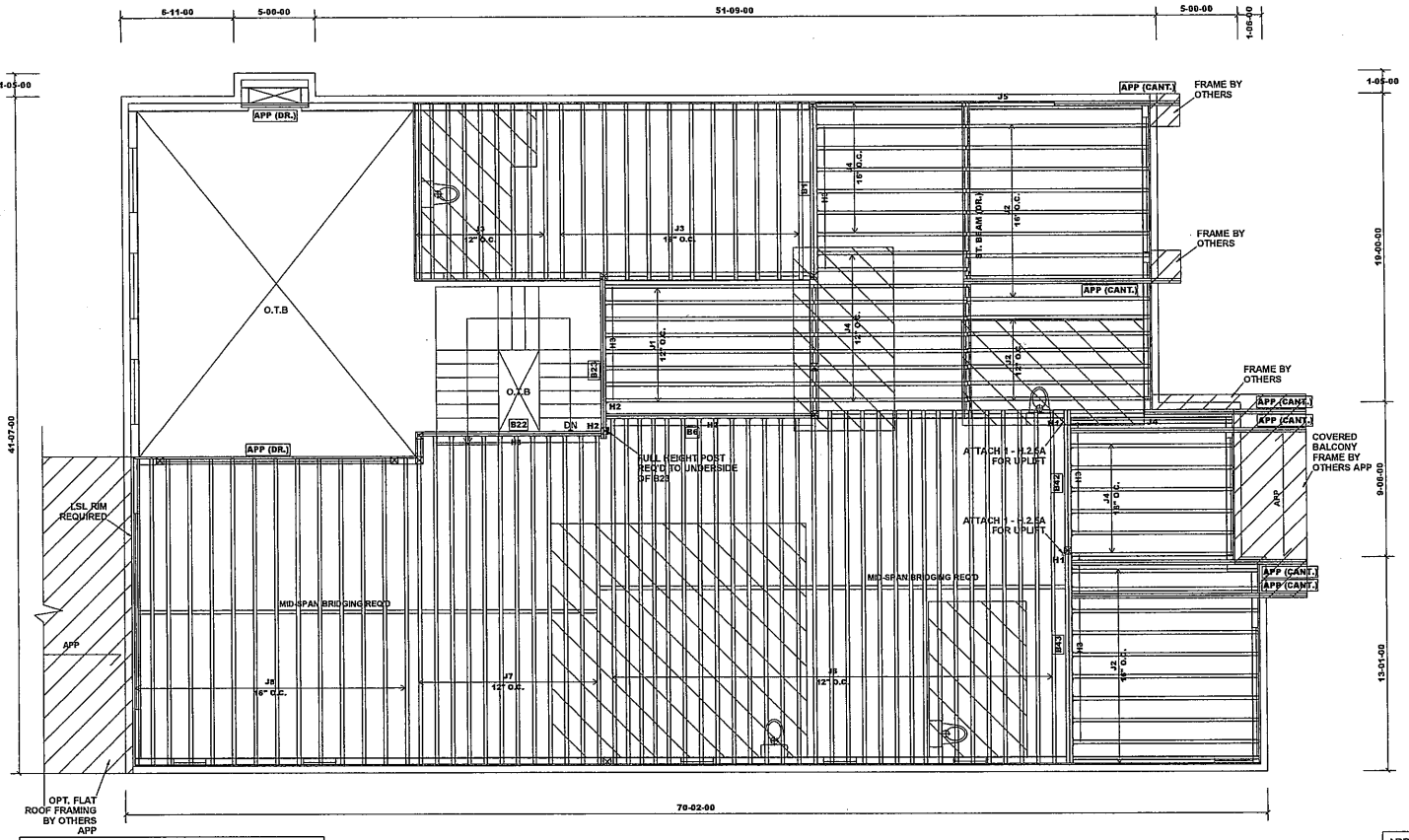
Builder: Gold Park Homes
Project: Pine Valley Ph2

Location: Vaughan, ON
Date: May. 06, 2022

Designer: TL
Sheet: 29 of 36

Alpa Roof Trusses Inc.
Stouffville, Ontario

Salesperson: Derek F.
Home Lumber Inc.



Products				
PlotID	Length	Product	Plies	Net Qty
B1	11-00-00	11 7/8" NI-20	2	2
B6	14-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B22	12-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B23	10-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B42	9-00-00	11 7/8" NI-20	2	2
B43	14-00-00	11 7/8" NI-20	2	2
J1	13-00-00	11 7/8" NI-20	1	8
J2	12-00-00	11 7/8" NI-20	1	25
J3	11-00-00	11 7/8" NI-20	1	21
J4	10-00-00	11 7/8" NI-20	1	24
J5	6-00-00	11 7/8" NI-20	1	1
J6	22-00-00	11 7/8" NI-40x	1	29
J7	21-00-00	11 7/8" NI-40x	1	12
J8	19-00-00	11 7/8" NI-40x	1	14
Ca1	190-00-00	1 1/8" x 11 7/8" Rim Board	1	1
Ca2	19-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	1	1
Bk1	93-00-00	11 7/8" NI-20	1	1

Connector Summary			
PlotID	Qty	Manuf	Product
H1	2	N/A	H2.5A
H2	2		HGUS410
H3	57		LT251188

SECOND FLOOR FRAMING
UNIT 5013 - THE RIVERVIEW
ELEVATION C
WO/ LOFT CONDITION

FLOOR LOADING
LIVE LOAD : 40 PSF
DEAD LOAD : 15 PSF
DEAD LOAD (TILE): 20 PSF

HATCH LEGEND
- Ceramic Tile
- Conv Framed

APP - AS PER PLAN
BBO - BEAM BY OTHERS
PA - POST ABOVE
O.T.B - OPEN TO BELOW
GT - GIRDER TRUSS
RT - ROOF TRUSS

RIMBOARD
1-1/8" X 11-7/8" O.S.B

SUBFLOOR: 3/4" NAILED & GLUED*

Blocking panels are required over all interior supports.
Squash blocks are required under concentrated loads.

Ceramic Tile Application as per O.B.C. 9.30.6

Provide I-Joist blocking between cantilevered joists (along bearing) and rimboard closure at ends.

Do not scale - refer to architectural plans for dimensions.

JT/PL: 45147/116460
LI: 343077*

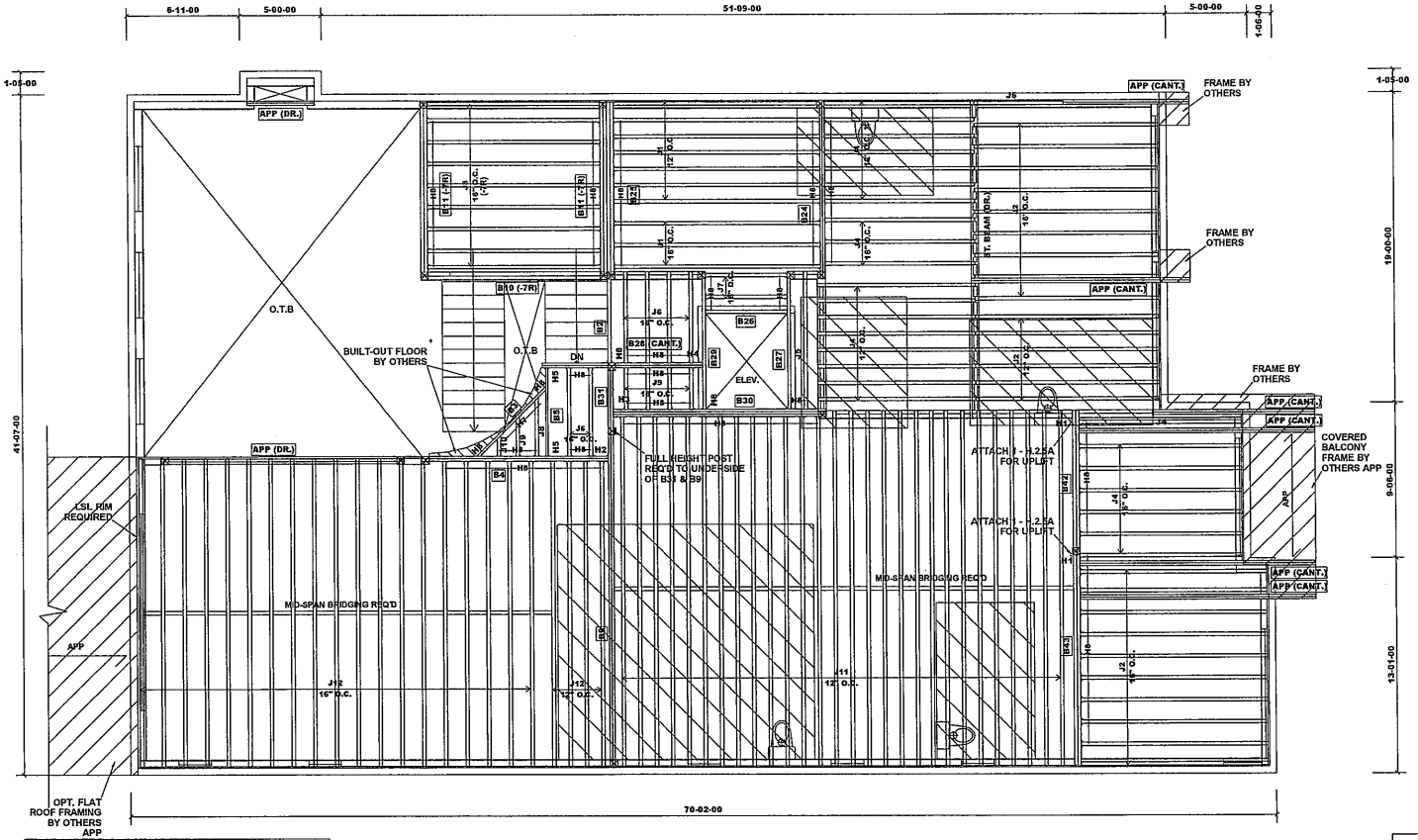
Builder: Gold Park Homes
Project: Pine Valley Ph2

Location: Vaughan, ON
Date: May. 06, 2022

Designer: TL
Sheet: 30 of 36

Alpa Roof Trusses Inc.
Stouffville, Ontario

Salesperson: Derek F.
Home Lumber Inc.



Products				
PlotID	Length	Product	Plies	Net Qty
B2	6-00-00	11 7/8" NI-20	1	1
B3	7-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	1	1
B4	12-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B5	6-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	1	1
B9	21-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B10 (-7R)	12-00-00	11 7/8" NI-20	1	1
B11 (-7R)	11-00-00	11 7/8" NI-20	2	4
B24	11-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B25	11-00-00	11 7/8" NI-20	2	2
B26	5-00-00	11 7/8" NI-20	1	1
B27	9-00-00	11 7/8" NI-20	1	1
B28 (CANT.)	10-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B29	9-00-00	11 7/8" NI-20	1	1
B30	14-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	3	3
B31	4-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B42	9-00-00	11 7/8" NI-20	2	2
B43	14-00-00	11 7/8" NI-20	2	2
J1	13-00-00	11 7/8" NI-20	1	10
J2	12-00-00	11 7/8" NI-20	1	25
J3	11-00-00	11 7/8" NI-20	1	9
J4	10-00-00	11 7/8" NI-20	1	25
J5	9-00-00	11 7/8" NI-20	1	1
J6	6-00-00	11 7/8" NI-20	1	7
J7	5-00-00	11 7/8" NI-20	1	2
J8	4-00-00	11 7/8" NI-20	1	1
J9	3-00-00	11 7/8" NI-20	1	5
J10	2-00-00	11 7/8" NI-20	1	1
J11	22-00-00	11 7/8" NI-40x	1	28
J12	19-00-00	11 7/8" NI-40x	1	23
Ca1	166-00-00	1 1/8" x 11 7/8" Rim Board	1	1
Ca2	19-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	1	1
Bk1	85-00-00	11 7/8" NI-20	1	1

Connector Summary			
PlotID	Qty	Manuf	Product
H1	2	N/A	H2.5A
H2	1		HGUS410
H3	1		HGUS5.50/10
H4	1		HU410
H5	2		HUS1.81/10
H6	2		LSSR1.81Z
H7	3		LSSR2.56Z
H8	110		LT251188

SECOND FLOOR FRAMING
UNIT 5013 - THE RIVERVIEW
ELEVATION C
W/ LOFT CONDITION
W/ ELEVATOR

FLOOR LOADING
LIVE LOAD : 40 PSF
DEAD LOAD : 15 PSF
DEAD LOAD (TILE): 20 PSF

HATCH LEGEND
- Ceramic Tile
- Conv Framed

APP - AS PER PLAN
BBO - BEAM BY OTHERS
PA - POST ABOVE
O.T.B - OPEN TO BELOW
GT - GIRDER TRUSS
RT - ROOF TRUSS

RIMBOARD
1-1/8" X 11-7/8" O.S.B

SUBFLOOR: 3/4" NAILED & GLUED*

Blocking panels are required over all interior supports.
Squash blocks are required under concentrated loads.

Ceramic Tile Application as per O.B.C. 9.30.6

Provide I-Joist blocking between cantilevered joists (along bearing) and rimboard closure at ends.

Do not scale - refer to architectural plans for dimensions.

JT/PL: 45147/116460
LI: 343077*

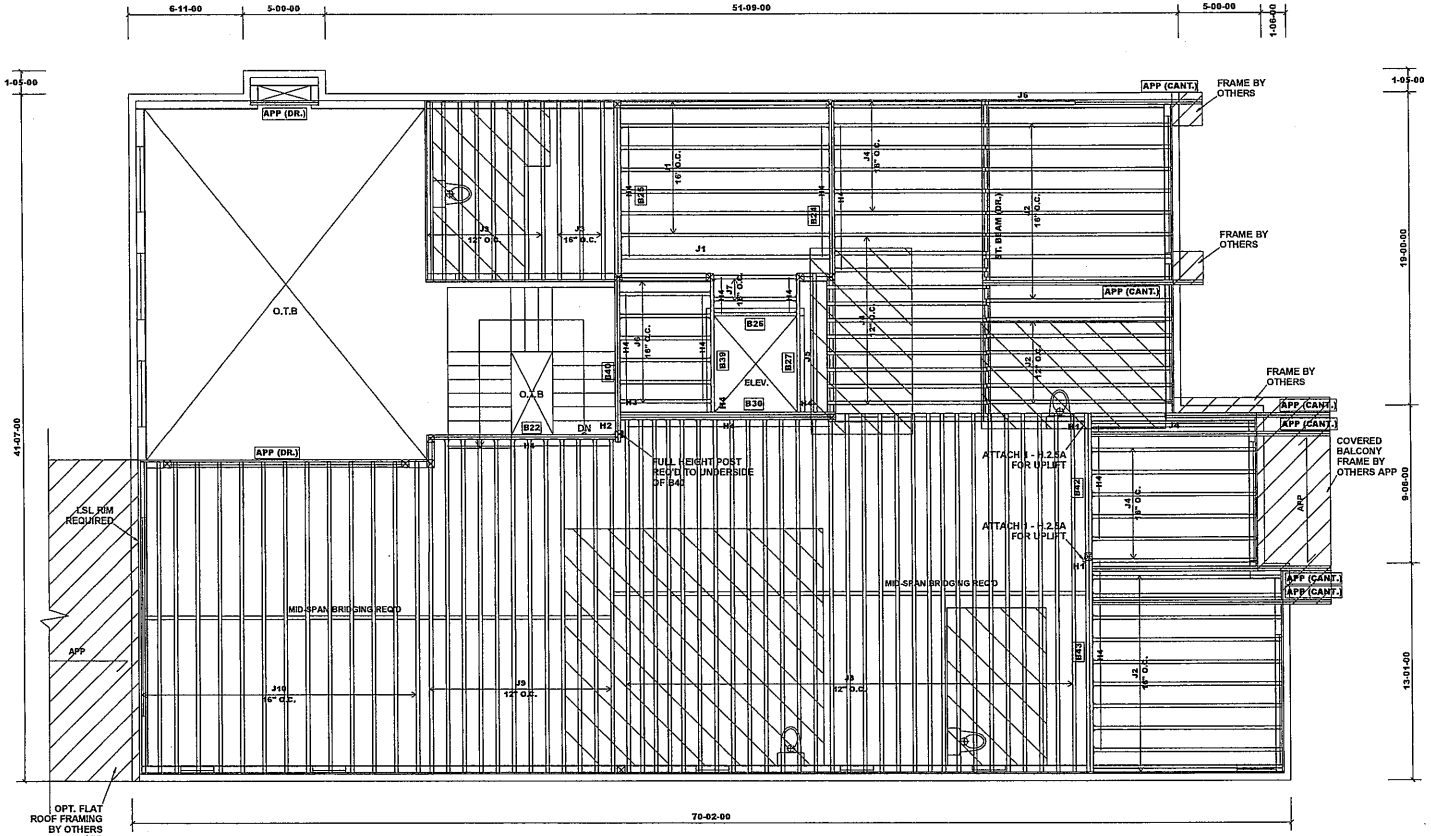
Builder: Gold Park Homes
Project: Pine Valley Ph2

Location: Vaughan, ON
Date: May. 06, 2022

Designer: TL
Sheet: 31 of 36

Alpa Roof Trusses Inc.
Stouffville, Ontario

Salesperson: Derek F.
Home Lumber Inc.



Products				
PlotID	Length	Product	Plies	Net Qty
B22	12-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B24	11-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B25	11-00-00	11 7/8" NI-20	2	2
B26	5-00-00	11 7/8" NI-20	1	1
B27	9-00-00	11 7/8" NI-20	1	1
B30	14-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	3	3
B39	9-00-00	11 7/8" NI-20	1	1
B40	10-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B42	9-00-00	11 7/8" NI-20	2	2
B43	14-00-00	11 7/8" NI-20	2	2
J1	13-00-00	11 7/8" NI-20	1	8
J2	12-00-00	11 7/8" NI-20	1	25
J3	11-00-00	11 7/8" NI-20	1	11
J4	10-00-00	11 7/8" NI-20	1	24
J5	9-00-00	11 7/8" NI-20	1	1
J6	6-00-00	11 7/8" NI-20	1	8
J7	5-00-00	11 7/8" NI-20	1	2
J8	22-00-00	11 7/8" NI-40x	1	29
J9	21-00-00	11 7/8" NI-40x	1	12
J10	19-00-00	11 7/8" NI-40x	1	14
Ca1	190-00-00	1 1/8" x 11 7/8" Rim Board	1	1
Ca2	16-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	1	1
Bk1	84-00-00	11 7/8" NI-20	1	1

Connector Summary			
PlotID	Qty	Manuf	Product
H1	2	N/A	H2.5A
H2	1		HGUS410
H3	1		HGUS5.50/10
H4	84		LT251188

SECOND FLOOR FRAMING
UNIT 5013 - THE RIVERVIEW
ELEVATION C
WO/ LOFT CONDITION
W/ ELEVATOR

FLOOR LOADING
LIVE LOAD : 40 PSF
DEAD LOAD : 15 PSF
DEAD LOAD (TILE): 20 PSF

HATCH LEGEND
Ceramic Tile
Conv Framed

APP - AS PER PLAN
BBO - BEAM BY OTHERS
PA - POST ABOVE
O.T.B - OPEN TO BELOW
GT - GIRDER TRUSS
RT - ROOF TRUSS

RIMBOARD
1-1/8" X 11-7/8" O.S.B

SUBFLOOR: 3/4" NAILED & GLUED*

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Builder: Gold Park Homes
Project: Pine Valley Ph2

Location: Vaughan, ON
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Sheet: 32 of 36

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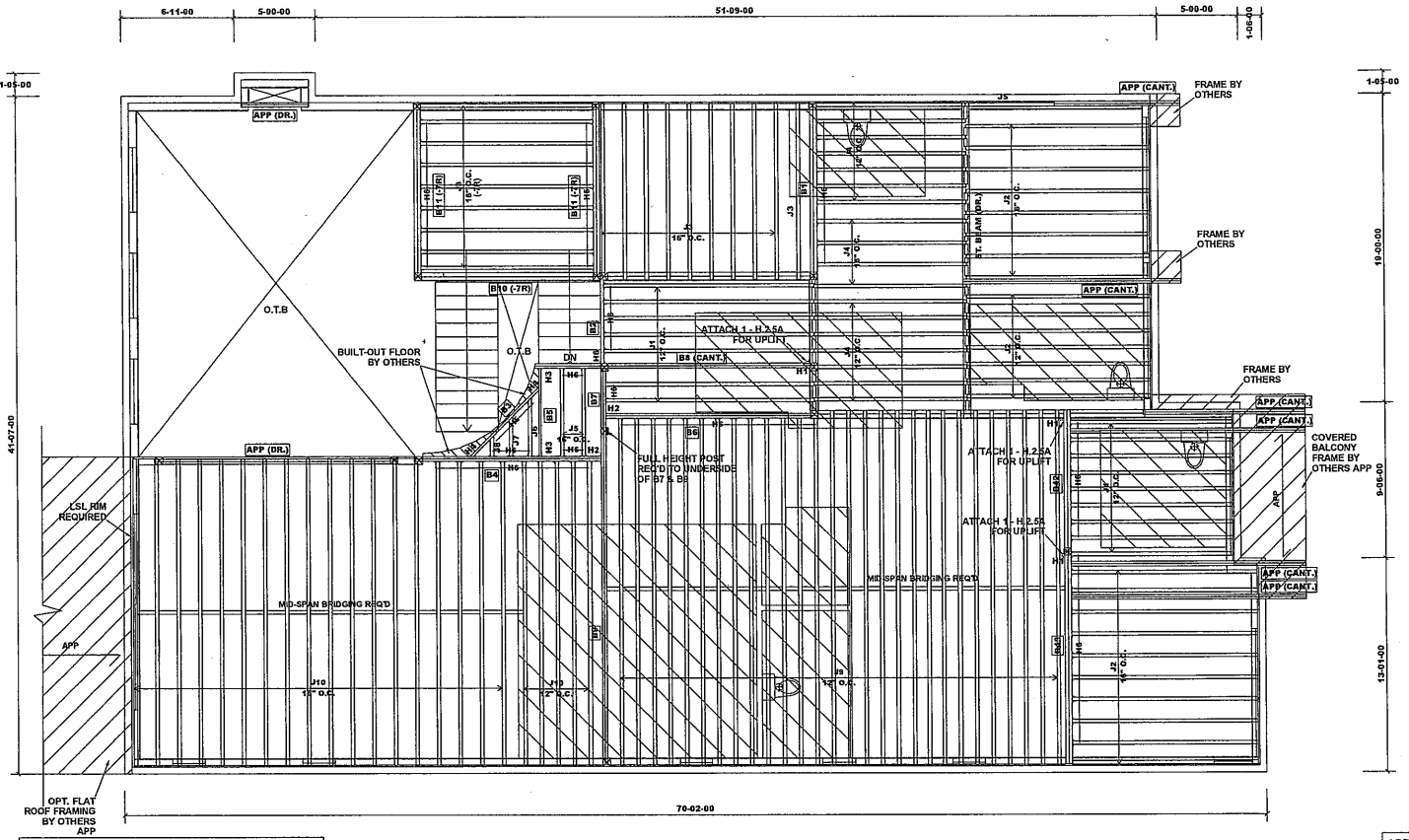


Products				
PlotID	Length	Product	Plies	Net Qty
B1	11-00-00	11 7/8" NI-20	2	2
B2	6-00-00	11 7/8" NI-20	1	1
B3	7-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	1	1
B4	12-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B5	6-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	1	1
B6	14-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B7	4-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B8 (CANT.)	18-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B9	21-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B10 (-7R)	12-00-00	11 7/8" NI-20	1	1
B11 (-7R)	11-00-00	11 7/8" NI-20	2	4
B42	9-00-00	11 7/8" NI-20	2	2
B43	14-00-00	11 7/8" NI-20	2	2
J1	13-00-00	11 7/8" NI-20	1	7
J2	12-00-00	11 7/8" NI-20	1	25
J3	11-00-00	11 7/8" NI-20	1	19
J4	10-00-00	11 7/8" NI-20	1	27
J5	6-00-00	11 7/8" NI-20	1	3
J6	4-00-00	11 7/8" NI-20	1	1
J7	3-00-00	11 7/8" NI-20	1	1
J8	2-00-00	11 7/8" NI-20	1	1
J9	22-00-00	11 7/8" NI-40x	1	28
J10	19-00-00	11 7/8" NI-40x	1	23
Ca1	176-00-00	1 1/8" x 11 7/8" Rim Board	1	1
Ca2	19-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	1	1
Bk1	88-00-00	11 7/8" NI-20	1	1

Connector Summary			
PlotID	Qty	Manuf	Product
H1	3	N/A	H2.5A
H2	2		HGUS410
H3	2		HUS1.81/10
H4	2		LSSR1.81Z
H5	3		LSSR2.56Z
H6	80		LT251188

SECOND FLOOR FRAMING
UNIT 5013 - THE RIVERVIEW
ELEVATION C
W/ LOFT CONDITION
W/ 5 BEDROOM

FLOOR LOADING
LIVE LOAD : 40 PSF
DEAD LOAD : 15 PSF
DEAD LOAD (TILE): 20 PSF

HATCH LEGEND
- Ceramic Tile
- Conv Framed

APP - AS PER PLAN
BBO - BEAM BY OTHERS
PA - POST ABOVE
O.T.B - OPEN TO BELOW
GT - GIRDER TRUSS
RT - ROOF TRUSS

RIMBOARD
1-1/8" X 11-7/8" O.S.B

SUBFLOOR: 3/4" NAILED & GLUED*

Blocking panels are required over all interior supports.
Squash blocks are required under concentrated loads.

Ceramic Tile Application as per O.B.C. 9.30.6

Provide I-Joist blocking between cantilevered joists (along bearing) and rimboard closure at ends.

Do not scale - refer to architectural plans for dimensions.

JT/PL: 45147/116460
LI: 343077*

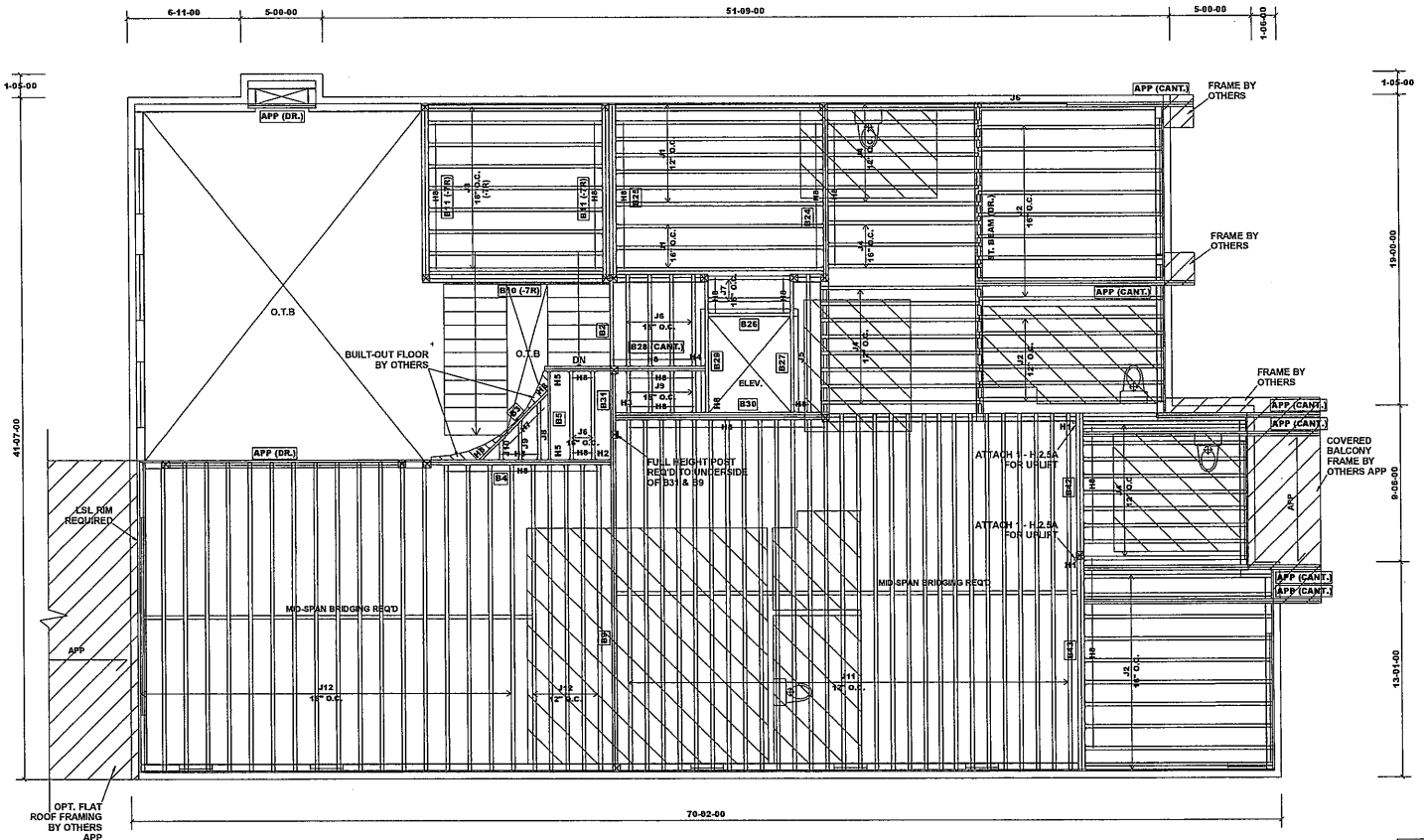
Builder: Gold Park Homes
Project: Pine Valley Ph2

Location: Vaughan, ON
Date: May. 06, 2022

Designer: TL
Sheet: 33 of 36

Alpa Roof Trusses Inc.
Stouffville, Ontario

Salesperson: Derek F.
Home Lumber Inc.



Products				
PlotID	Length	Product	Plies	Net Qty
B2	6-00-00	11 7/8" NI-20	1	1
B3	7-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	1	1
B4	12-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B5	5-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	1	1
B9	21-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B10 (-7R)	12-00-00	11 7/8" NI-20	1	1
B11 (-7R)	11-00-00	11 7/8" NI-20	2	4
B24	11-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B25	11-00-00	11 7/8" NI-20	2	2
B26	5-00-00	11 7/8" NI-20	1	1
B27	9-00-00	11 7/8" NI-20	1	1
B28 (CANT.)	10-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B29	9-00-00	11 7/8" NI-20	1	1
B30	14-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	3	3
B31	4-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B42	9-00-00	11 7/8" NI-20	2	2
B43	14-00-00	11 7/8" NI-20	2	2
J1	13-00-00	11 7/8" NI-20	1	10
J2	12-00-00	11 7/8" NI-20	1	25
J3	11-00-00	11 7/8" NI-20	1	9
J4	10-00-00	11 7/8" NI-20	1	27
J5	9-00-00	11 7/8" NI-20	1	1
J6	6-00-00	11 7/8" NI-20	1	7
J7	5-00-00	11 7/8" NI-20	1	2
J8	4-00-00	11 7/8" NI-20	1	1
J9	3-00-00	11 7/8" NI-20	1	5
J10	2-00-00	11 7/8" NI-20	1	1
J11	22-00-00	11 7/8" NI-40x	1	28
J12	19-00-00	11 7/8" NI-40x	1	23
Ca1	166-00-00	1 1/8" x 11 7/8" Rim Board	1	1
Ca2	19-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	1	1
Bk1	83-00-00	11 7/8" NI-20	1	1

Connector Summary			
PlotID	Qty	Manuf	Product
H1	2	N/A	H2.5A
H2	1		HGUS410
H3	1		HGUS5.50/10
H4	1		HU410
H5	2		HUS1.81/10
H6	2		LSSR1.81Z
H7	3		LSSR2.56Z
H8	112		LTZ51188

SECOND FLOOR FRAMING
UNIT 5013 - THE RIVERVIEW
ELEVATION C
W/ LOFT CONDITION
W/ ELEVATOR
W/ 5 BEDROOM

FLOOR LOADING
LIVE LOAD : 40 PSF
DEAD LOAD : 15 PSF
DEAD LOAD (TILE): 20 PSF

HATCH LEGEND
Ceramic Tile
Conv Framed

APP - AS PER PLAN
BBO - BEAM BY OTHERS
PA - POST ABOVE
O.T.B - OPEN TO BELOW
GT - GIRDER TRUSS
RT - ROOF TRUSS

RIMBOARD
1-1/8" X 11-7/8" O.S.B

SUBFLOOR: 3/4" NAILED & GLUED*

Blocking panels are required over all interior supports.
Squash blocks are required under concentrated loads.

Ceramic Tile Application as per O.B.C. 9.30.6

Provide I-Joist blocking between cantilevered joists (along bearing) and rimboard closure at ends.

Do not scale - refer to architectural plans for dimensions.

JT/PL: 45147/116460
LI: 343077*

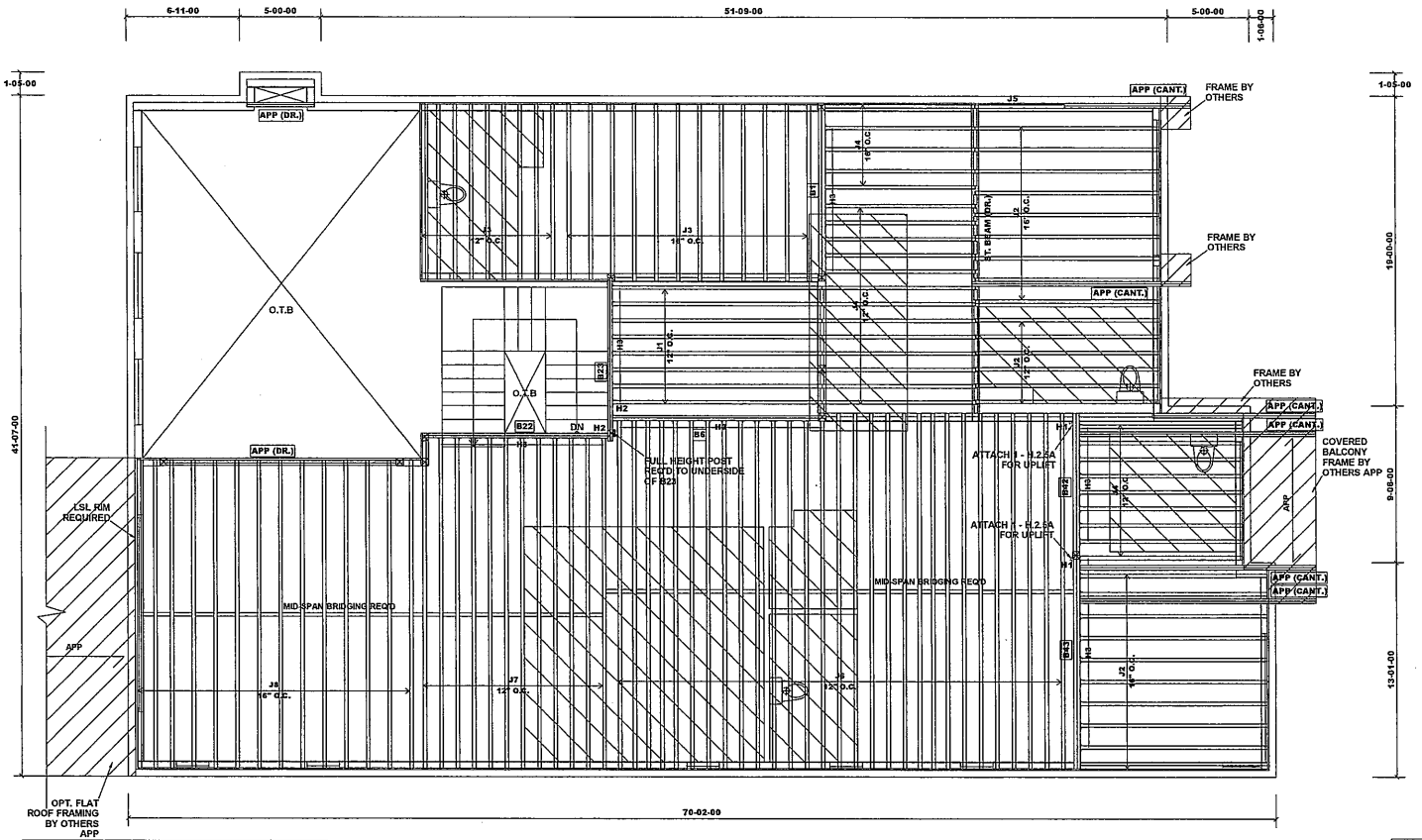
Builder: Gold Park Homes
Project: Pine Valley Ph2

Location: Vaughan, ON
Date: May. 06, 2022

Designer: TL
Sheet: 34 of 36

Alpa Roof Trusses Inc.
Stouffville, Ontario

Salesperson: Derek F.
Home Lumber Inc.



Products				
PlotID	Length	Product	Plies	Net Qty
B1	11-00-00	11 7/8" NI-20	2	2
B6	14-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B22	12-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B23	10-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B42	9-00-00	11 7/8" NI-20	2	2
B43	14-00-00	11 7/8" NI-20	2	2
J1	13-00-00	11 7/8" NI-20	1	8
J2	12-00-00	11 7/8" NI-20	1	25
J3	11-00-00	11 7/8" NI-20	1	21
J4	10-00-00	11 7/8" NI-20	1	27
J5	6-00-00	11 7/8" NI-20	1	1
J6	22-00-00	11 7/8" NI-40x	1	29
J7	21-00-00	11 7/8" NI-40x	1	12
J8	19-00-00	11 7/8" NI-40x	1	14
Ca1	190-00-00	1 1/8" x 11 7/8" Rim Board	1	1
Ca2	19-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	1	1
Bk1	93-00-00	11 7/8" NI-20	1	1

Connector Summary			
PlotID	Qty	Manuf	Product
H1	2	N/A	H2.5A
H2	2		HGUS410
H3	60		LT251188

SECOND FLOOR FRAMING
UNIT 5013 - THE RIVERVIEW
ELEVATION C
W/O LOFT CONDITION
W/ 5 BEDROOM

FLOOR LOADING
LIVE LOAD : 40 PSF
DEAD LOAD : 15 PSF
DEAD LOAD (TILE): 20 PSF

HATCH LEGEND
- Ceramic Tile
- Conv Framed

APP - AS PER PLAN
BBO - BEAM BY OTHERS
PA - POST ABOVE
O.T.B - OPEN TO BELOW
GT - GIRDER TRUSS
RT - ROOF TRUSS

RIMBOARD
1-1/8" X 11-7/8" O.S.B

SUBFLOOR: 3/4" NAILED & GLUED*

Blocking panels are required over all interior supports.
Squash blocks are required under concentrated loads.

Ceramic Tile Application as per O.B.C. 9.30.6

Provide I-Joist blocking between cantilevered joists (along bearing) and rimboard closure at ends.

Do not scale - refer to architectural plans for dimensions.

JT/PL: 45147/116460
LI: 343077*

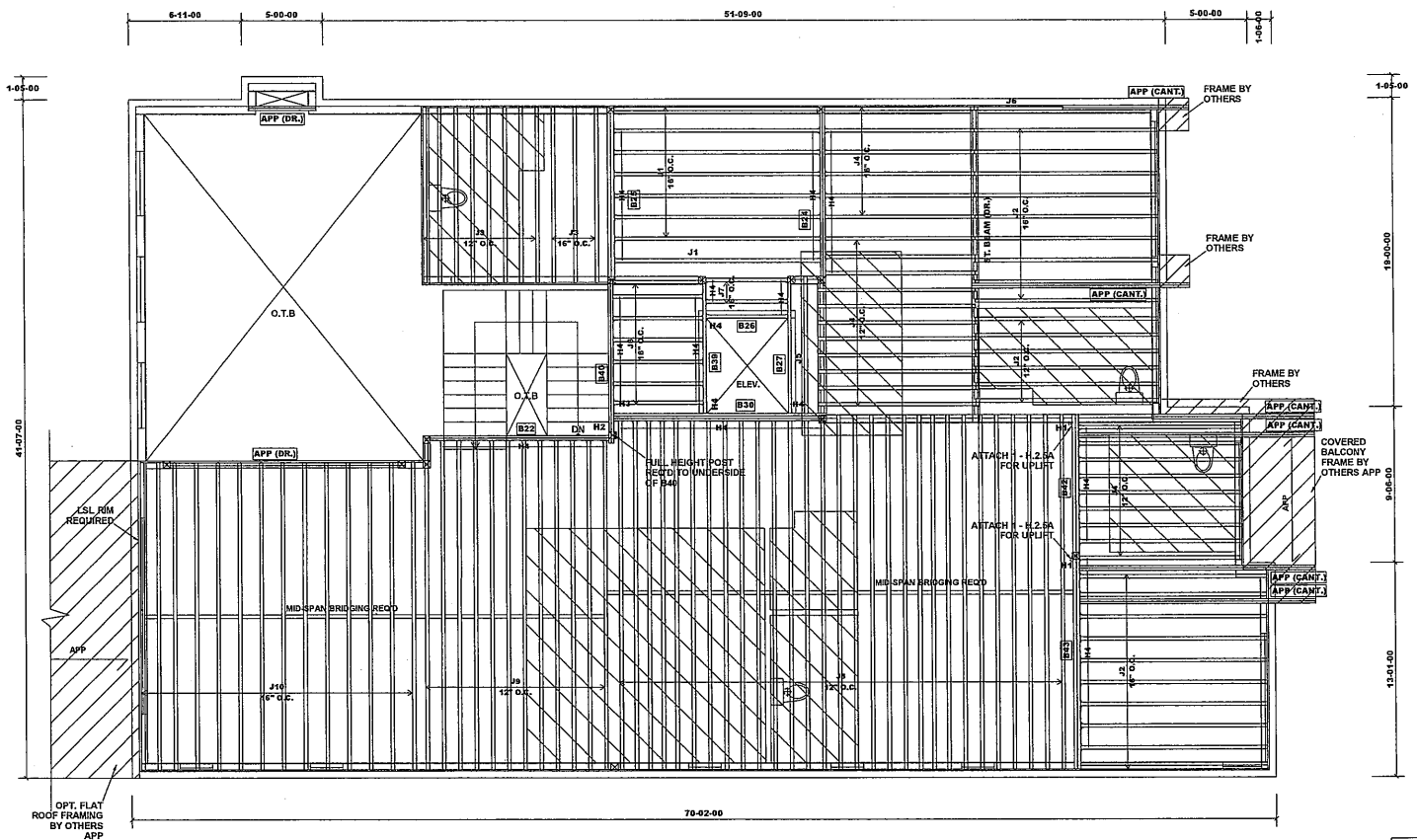
Builder: Gold Park Homes
Project: Pine Valley Ph2

Location: Vaughan, ON
Date: May. 06, 2022

Designer: TL
Sheet: 35 of 36

Alpa Roof Trusses Inc.
Stouffville, Ontario

Salesperson: Derek F.
Home Lumber Inc.



Products				
PlotID	Length	Product	Plies	Net Qty
B22	12-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B24	11-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B25	11-00-00	11 7/8" NI-20	2	2
B26	5-00-00	11 7/8" NI-20	1	1
B27	9-00-00	11 7/8" NI-20	1	1
B30	14-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	3	3
B39	9-00-00	11 7/8" NI-20	1	1
B40	10-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	2	2
B42	9-00-00	11 7/8" NI-20	2	2
B43	14-00-00	11 7/8" NI-20	2	2
J1	13-00-00	11 7/8" NI-20	1	8
J2	12-00-00	11 7/8" NI-20	1	25
J3	11-00-00	11 7/8" NI-20	1	11
J4	10-00-00	11 7/8" NI-20	1	28
J5	9-00-00	11 7/8" NI-20	1	1
J6	6-00-00	11 7/8" NI-20	1	8
J7	5-00-00	11 7/8" NI-20	1	2
J8	22-00-00	11 7/8" NI-40x	1	29
J9	21-00-00	11 7/8" NI-40x	1	12
J10	19-00-00	11 7/8" NI-40x	1	14
Ca1	190-00-00	1 1/8" x 11 7/8" Rim Board	1	1
Ca2	19-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand® LSL	1	1
Blk1	84-00-00	11 7/8" NI-20	1	1

Connector Summary			
PlotID	Qty	Manuf	Product
H1	2	N/A	H2.5A
H2	1		HGUS410
H3	1		HGUS5.50/10
H4	86		LT25188

SECOND FLOOR FRAMING
UNIT 5013 - THE RIVERVIEW
ELEVATION C
WO/ LOFT CONDITION
W/ ELEVATOR
W/ 5 BEDROOM

FLOOR LOADING
LIVE LOAD : 40 PSF
DEAD LOAD : 15 PSF
DEAD LOAD (TILE): 20 PSF

HATCH LEGEND
Ceramic Tile
Conv Framed

APP - AS PER PLAN
BBO - BEAM BY OTHERS
PA - POST ABOVE
O.T.B - OPEN TO BELOW
GT - GIRDER TRUSS
RT - ROOF TRUSS

RIMBOARD
1-1/8" X 11-7/8" O.S.B

SUBFLOOR: 3/4" NAILED & GLUED*

Blocking panels are required over all interior supports.
Squash blocks are required under concentrated loads.

Ceramic Tile Application as per O.B.C. 9.30.6

Provide I-Joist blocking between cantilevered joists (along bearing) and rimboard closure at ends.

Do not scale - refer to architectural plans for dimensions.

JT/PL: 45147/116460
LI: 343077*

Builder: Gold Park Homes
Project: Pine Valley Ph2

Location: Vaughan, ON
Date: May. 06, 2022

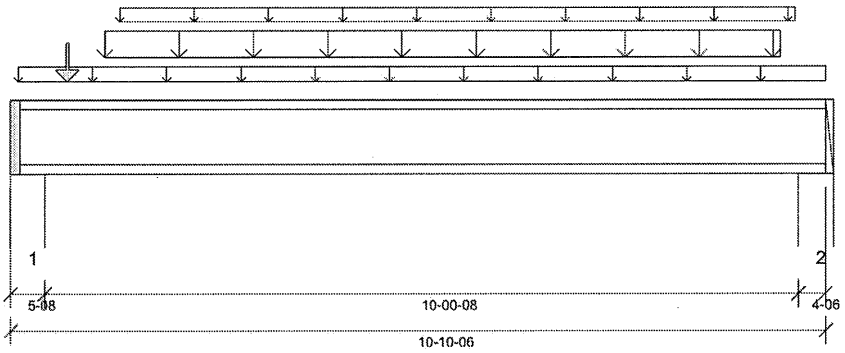
Designer: TL
Sheet: 36 of 36

Alpa Roof Trusses Inc.
Stouffville, Ontario

Salesperson: Derek F.
Home Lumber Inc.

MiTek	Customer: **Gold Park Homes** Job Address: **Pine Valley Ph2** City: **Vaughan** Job Track: **45147**	Job Name: **343077 Ground A + Second A (1** Level: **Second Floor** Label: **B1 - i50389** Type: **Beam**	**2 Ply Member** **11 7/8" NI-20**	Status: **Design Passed**

Illustration Not to Scale. Pitch: 0/12 | Designed by Single Member Design Engine in MiTek® Structure Version 8.5.3.233.Update5.15 | Report Version: 2021.03.26 05/02/2022 17:05



DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)
Design Methodology: LSD
Service Condition: Dry
LL Deflection Limit: L/360,
TL Deflection Limit: L/240,

Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 0'- 9 1/2"

Factored Resistance of Support Material:

- 615 psi Wall @ 0'- 4 1/2"
- 615 psi Wall @ 10'- 7"

REGISTERED PROFESSIONAL ENGINEER
06/02/2022
EDWIN C. FOK
PROVINCE OF ONTARIO

ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	5'- 9 1/8"	1.25D + 1.5L	1.00	5751 lb ft	11160 lb ft	Passed - 52%
Factored Shear:	0'- 5 9/16"	1.25D + 1.5L	1.00	2310 lb	4480 lb	Passed - 52%
Live Load (LL) Pos. Defl.:	5'- 5 3/4"	L		0.122"	L/360	Passed - L/990
Total Load (TL) Pos. Defl.:	5'- 5 3/4"	D + L		0.182"	L/240	Passed - L/663

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	5-08	1.25D + 1.5L	1.00	2326 lb		4480 lb	16918 lb	Passed - 52%
2	4-06	1.25D + 1.5L	1.00	2135 lb		4480 lb	13458 lb	Passed - 48%

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	10'- 10 3/8"	Self Weight	Top	6 lb/ft	-	-	-
Uniform	0'- 1 1/4"	10'- 10 3/8"	FC3 Floor Decking (Plan View Fill)	Top	7 lb/ft	18 lb/ft	-	-
Uniform	1'- 3 1/8"	10'- 3 1/8"	Smoothed Load	Front	87 lb/ft	189 lb/ft	-	-
Uniform	1'- 5 1/2"	10'- 5 1/2"	FC3 Floor Decking (Plan View Fill)	Top	2 lb/ft	-	-	-
Point	0'- 9 1/8"	0'- 9 1/8"	J4(i50378)	Front	82 lb	216 lb	-	-

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 5 1/2"	9(i41702)	528 lb	1111 lb	-	-
2	10'- 6"	10'- 10 3/8"	E25(i41701)	500 lb	1007 lb	-	-

DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.

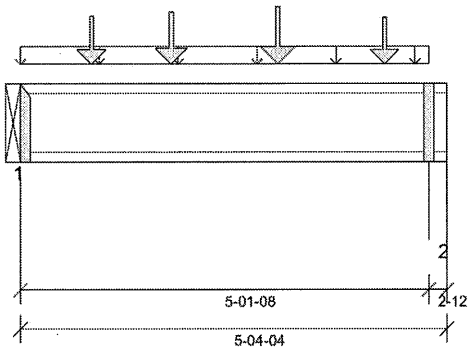
PLY TO PLY CONNECTION

- Member design assumed proper ply to ply connection by others. Verify connection between plies according to code specification and follow the manufacturer's installation instruction. Loads assumed to be distributed equally to each ply.

SE046658

MiTek	Customer: **Gold Park Homes** Job Address: **Pine Valley Ph2** City: **Vaughan** Job Track: **45147**	Job Name: **343077 Ground A + Second A (1,** Level: **Second Floor** Label: **B2 - i50409** Type: **Beam**	**1 Ply Member** **11 7/8" NI-20**	Status: **Design Passed**

Illustration Not to Scale. Pitch: 0/12 | Designed by Single Member Design Engine in MiTek® Structure Version 8.5.3.233.Update5.15 | Report Version: 2021.03.26 05/02/2022 17:06



DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)
Design Methodology: LSD
Service Condition: Dry
LL Deflection Limit: L/360,
TL Deflection Limit: L/240,

Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 1'- 1 1/2"

Factored Resistance of Support Material:

- 769 psi Beam @ 0'
- 615 psi Wall @ 5'- 2 1/2"

ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	2'- 9 9/16"	1.25D + 1.5L	1.00	1932 lb ft	5580 lb ft	Passed - 35%
Factored Shear:	5'- 1 7/16"	1.25D + 1.5L	1.00	1392 lb	2240 lb	Passed - 62%
Live Load (LL) Pos. Defl.:	2'- 7 1/8"	L		0.030"	L/360	Passed - L/999
Total Load (TL) Pos. Defl.:	2'- 7 3/16"	D + L		0.048"	L/240	Passed - L/999

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	1-12	1.25D + 1.5L	1.00	1359 lb		1970 lb	-	Passed - 69%
2	2-12	1.25D + 1.5L	1.00	1396 lb		2090 lb	4230 lb	Passed - 67%

CONNECTOR INFORMATION

ID	Part No.	Manufacturer	Nailing Requirements			Other Information or Requirement for Reinforcement Accessories
			Top	Face	Member	
1	LT251188		-	-	-	Connector manually specified by the user.

* Connectors: Refer to manufacturer's specifications, fasteners requirements and installation instruction. Where header fasteners are longer than the width of the supporting member, install backer block or clinch header nails.

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	5'- 4 1/4"	Self Weight	Top	3 lb/ft	-	-	-
Uniform	-0'	5'- 1 1/2"	User Load	Top	60 lb/ft	-	-	-
Point	0'- 10 3/4"	0'- 10 3/4"	J2(i50074)	Front	105 lb	269 lb	-	-
Point	1'- 10 3/4"	1'- 10 3/4"	J2(i50075)	Front	115 lb	307 lb	-	-
Point	3'- 2 3/4"	3'- 2 3/4"	J2(i50504)	Front	132 lb	351 lb	-	-
Point	4'- 6 3/4"	4'- 6 3/4"	J2(i50340)	Front	99 lb	263 lb	-	-

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'	B8 (CANT.)(i50483)	386 lb	584 lb	-	-
2	5'- 1 1/2"	5'- 4 1/4"	9(i41702)	388 lb	607 lb	-	-

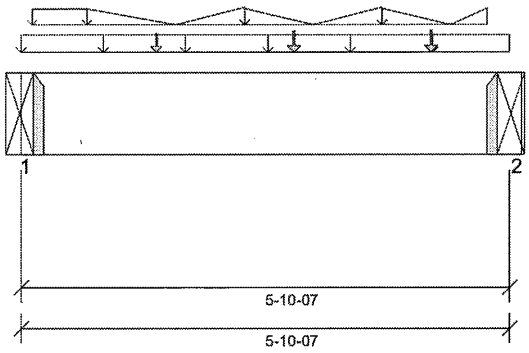
DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.

SG046659

MiTek	Customer: **Gold Park Homes** Job Address: **Pine Valley Ph2** City: **Vaughan** Job Track: **45147**	Job Name: **343077 Ground A + Second A (1** Level: **Second Floor** Label: **B3 - i50326** Type: **Beam**	**1 Ply Member** **1 3/4" x 11 7/8" 1.55E TimberStrand® LSL**	Status: **Design Passed**

Illustration Not to Scale. Pitch: 0/12 | Designed by Single Member Design Engine in MiTek® Structure Version 8.5.3.233.Update5.15 | Report Version: 2021.03.26 05/02/2022 17:06



DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)
Design Methodology: LSD
Service Condition: Dry
LL Deflection Limit: L/360,
TL Deflection Limit: L/240,

Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0'- 1 3/4" Bottom: 1'- 5 1/8"

Factored Resistance of Support Material:

- 769 psi Beam @ 0'
- 769 psi Beam @ 5'- 10 7/16"

REGISTERED PROFESSIONAL ENGINEER
06/02/2022
EDWIN C. FOK
PROVINCE OF ONTARIO

ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	3'- 3 3/8"	1.25D + 1.5L	1.00	585 lb ft	13266 lb ft	Passed - 4%
Factored Shear:	0'- 11 7/8"	1.25D + 1.5L	1.00	256 lb	7207 lb	Passed - 4%

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	1-08	1.25D + 1.5L	1.00	332 lb		3440 lb	-	Passed - 10%
2	1-08	1.25D + 1.5L	1.00	397 lb		3440 lb	-	Passed - 12%

CONNECTOR INFORMATION

ID	Part No.	Manufacturer	Nailing Requirements			Other Information or Requirement for Reinforcement Accessories
			Top	Face	Member	
1	LSSR1.81Z		-	-	-	Connector manually specified by the user.
2	LSSR1.81Z		-	-	-	Connector manually specified by the user.

* Connectors: Refer to manufacturer's specifications, fasteners requirements and installation instruction. Where header fasteners are longer than the width of the supporting member, install backer block or clinch header nails.

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	-0'	5'- 10 7/16"	Self Weight	Top	6 lb/ft	-	-	-
Uniform	-0'	5'- 10 7/16"	User Load	Top	10 lb/ft	27 lb/ft	-	-
Tapered	0'- 1 5/8"	0'- 9 1/2"	FC3 Floor Decking (Plan View Fill)	Top	2 To 4 lb/ft	4 To 11 lb/ft	-	-
Tapered	0'- 9 1/2"	1'- 10 3/8"	FC3 Floor Decking (Plan View Fill)	Top	4 To 0 lb/ft	11 To 0 lb/ft	-	-
Tapered	1'- 10 3/8"	2'- 8 1/4"	FC3 Floor Decking (Plan View Fill)	Top	0 To 12 lb/ft	0 To 33 lb/ft	-	-
Tapered	2'- 8 1/4"	3'- 6 3/16"	FC3 Floor Decking (Plan View Fill)	Top	6 To 0 lb/ft	17 To 0 lb/ft	-	-
Tapered	3'- 6 3/16"	4'- 4 1/16"	FC3 Floor Decking (Plan View Fill)	Top	0 To 12 lb/ft	0 To 33 lb/ft	-	-
Tapered	4'- 4 1/16"	5'- 1 15/16"	FC3 Floor Decking (Plan View Fill)	Top	6 To 0 lb/ft	17 To 0 lb/ft	-	-
Tapered	5'- 1 15/16"	5'- 7 1/4"	FC3 Floor Decking (Plan View Fill)	Top	0 To 7 lb/ft	0 To 18 lb/ft	-	-
Point	1'- 7 9/16"	1'- 7 9/16"	J8(i50020)	Front	10 lb	28 lb	-	-
Point	3'- 3 3/8"	3'- 3 3/8"	J7(i50303)	Front	18 lb	49 lb	-	-
Point	4'- 11 1/8"	4'- 11 1/8"	J6(i50022)	Front	20 lb	53 lb	-	-

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	-0'	0'- 1 3/4"	B4(i49924)	76 lb	153 lb	-	-
2	5'- 8 11/16"	5'- 10 7/16"	B5(i50292)	90 lb	192 lb	-	-

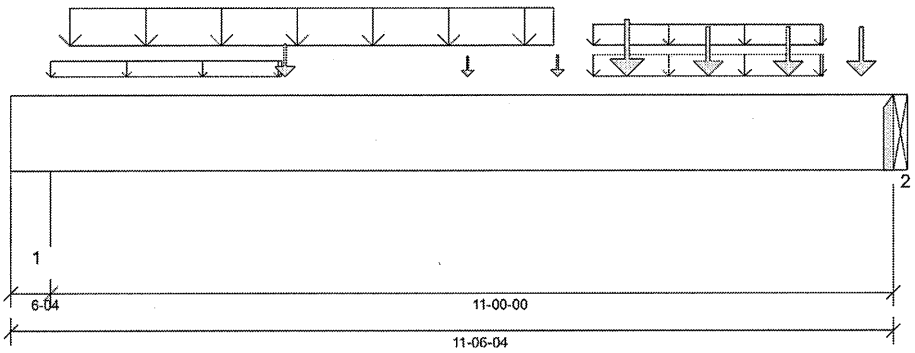
DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.

1046660

MiTek	Customer: **Gold Park Homes** Job Address: **Pine Valley Ph2** City: **Vaughan** Job Track: **45147**	Job Name: **343077 Ground A + Second A (1,** Level: **Second Floor** Label: **B4 - i49924** Type: **Beam**	**2 Ply Member** **1 3/4" x 11 7/8" 1.55E TimberStrand® LSL**	Status: **Design Passed**

Illustration Not to Scale. Pitch: 0/12 — Designed by Single Member Design Engine in MiTek® Structure Version 8.5.3.233.Update5.15 — Report Version: 2021.03.26 05/02/2022 17:07



DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)
Design Methodology: LSD
Service Condition: Dry
LL Deflection Limit: L/360,
TL Deflection Limit: L/240,

Lateral Restraint Requirements:

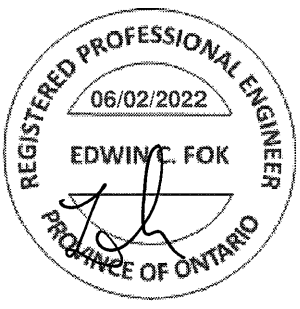
Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 1'- 1 1/2"

Factored Resistance of Support Material:

- 615 psi Wall @ 0'- 5 1/4"
- 769 psi Beam @ 11'- 6 1/4"

NAIL ONE PLY TO ANOTHER WITH 3-1/2" SPIRAL NAILS @9 " O/C STAGGERED IN 2 ROWS



ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	5'- 11 5/8"	1.25D + 1.5L	1.00	14090 lb ft	26531 lb ft	Passed - 53%
Factored Shear:	10'- 6 3/8"	1.25D + 1.5L	1.00	4544 lb	14414 lb	Passed - 32%
Live Load (LL) Pos. Defl.:	6'	L		0.211"	L/360	Passed - L/627
Total Load (TL) Pos. Defl.:	6'- 1/16"	D + L		0.300"	L/240	Passed - L/439

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	6-04	1.25D + 1.5L	1.00	4683 lb		28665 lb	13458 lb	Passed - 35%
2	1-08	1.25D + 1.5L	1.00	5160 lb		6880 lb	-	Passed - 75%

CONNECTOR INFORMATION

ID	Part No.	Manufacturer	Nailing Requirements			Other Information or Requirement for Reinforcement Accessories
			Top	Face	Member	
2	HGUS410		-	-	-	Connector manually specified by the user.

* Connectors: Refer to manufacturer's specifications, fasteners requirements and installation instruction. Where header fasteners are longer than the width of the supporting member, install backer block or clinch header nails.

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	11'- 6 1/4"	Self Weight	Top	13 lb/ft	-	-	-
Uniform	0'- 6 1/4"	3'- 6 7/16"	User Load	Top	10 lb/ft	27 lb/ft	-	-
Uniform	0'- 9 1/4"	7'- 1 1/16"	Smoothed Load	Front	148 lb/ft	395 lb/ft	-	-
Uniform	7'- 7 1/4"	10'- 7 1/4"	Smoothed Load	Front	169 lb/ft	-	-	-
Tapered	7'- 7 1/4"	10'- 7 1/4"	Smoothed Load	Back	-	163 To 146 lb/ft	-	-
Point	8'- 9/16"	8'- 9/16"	-	Front	84 lb	434 lb	-	-
Point	9'- 1 5/16"	9'- 1 5/16"	-	Front	58 lb	372 lb	-	-
Point	10'- 1 13/16"	10'- 1 13/16"	-	Front	54 lb	372 lb	-	-
Point	11'- 1 1/4"	11'- 1 1/4"	J10(i50070)	Front	131 lb	291 lb	-	-
Point	3'- 6 15/16"	3'- 6 15/16"	B3(i50326)	Back	76 lb	153 lb	-	-
Point	5'- 11 5/8"	5'- 11 5/8"	J7(i50303)	Back	21 lb	56 lb	-	-
Point	7'- 1 5/8"	7'- 1 5/8"	J6(i50022)	Back	24 lb	64 lb	-	-

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 6 1/4"	7(i41696)	977 lb	2311 lb	-	-
2	11'- 6 1/4"	11'- 6 1/4"	B9(i49806)	1107 lb	2514 lb	-	-

DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Lateral stability factor (KL) was based on user preference to use the width of one ply.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.

PLY TO PLY CONNECTION

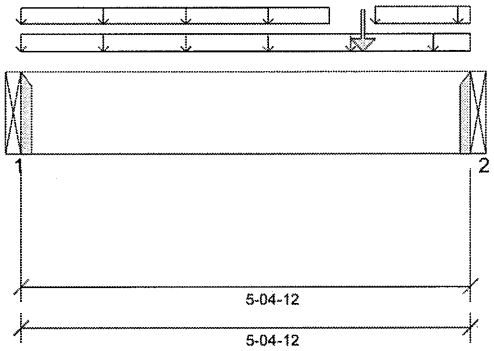
SG046661

MiTek	Customer: **Gold Park Homes** Job Address: **Pine Valley Ph2** City: **Vaughan** Job Track: **45147**	Job Name: **343077 Ground A + Second A (1,** Level: **Second Floor** Label: **B5 - i50292** Type: **Beam**	**1 Ply Member** **1 3/4" x 11 7/8" 1.55E TimberStrand® LSL**	Status: **Design Passed**

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in MiTek® Structure Version 8.5.3.233.Update5.15

Report Version: 2021.03.26 05/02/2022 17:07



DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)
Design Methodology: LSD
Service Condition: Dry
LL Deflection Limit: L/360,
TL Deflection Limit: L/240,

Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 4'- 9/16"

Factored Resistance of Support Material:

- 769 psi Beam @ 0'
- 769 psi Beam @ 5'- 4 3/4"

ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	3'- 10 1/16"	1.25D + 1.5L	1.00	630 lb ft	13266 lb ft	Passed - 5%
Factored Shear:	4'- 4 7/8"	1.25D + 1.5L	1.00	446 lb	7207 lb	Passed - 6%

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	1-08	1.25D + 1.5L	1.00	342 lb		3440 lb	-	Passed - 10%
2	1-08	1.25D + 1.5L	1.00	579 lb		3440 lb	-	Passed - 17%

CONNECTOR INFORMATION

ID	Part No.	Manufacturer	Nailing Requirements			Other Information or Requirement for Reinforcement Accessories
			Top	Face	Member	
1	HUS1.81/10		-	-	-	Connector manually specified by the user.
2	HUS1.81/10		-	-	-	Connector manually specified by the user.

* Connectors: Refer to manufacturer's specifications, fasteners requirements and installation instruction. Where header fasteners are longer than the width of the supporting member, install backer block or clinch header nails.

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	5'- 4 3/4"	Self Weight	Top	6 lb/ft	-	-	-
Uniform	0'	5'- 4 3/4"	-	Top	10 lb/ft	27 lb/ft	-	-
Uniform	0'	3'- 8 7/16"	FC3 Floor Decking (Plan View Fill)	Top	5 lb/ft	12 lb/ft	-	-
Uniform	4'- 3 1/16"	5'- 4 3/4"	FC3 Floor Decking (Plan View Fill)	Top	11 lb/ft	29 lb/ft	-	-
Point	4'- 1 5/16"	4'- 1 5/16"	B3(i50326)	Back	90 lb	192 lb	-	-

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'	B4(i49924)	84 lb	165 lb	-	-
2	5'- 4 3/4"	5'- 4 3/4"	B8 (CANT.)(i50483)	132 lb	270 lb	-	-

DESIGN NOTES

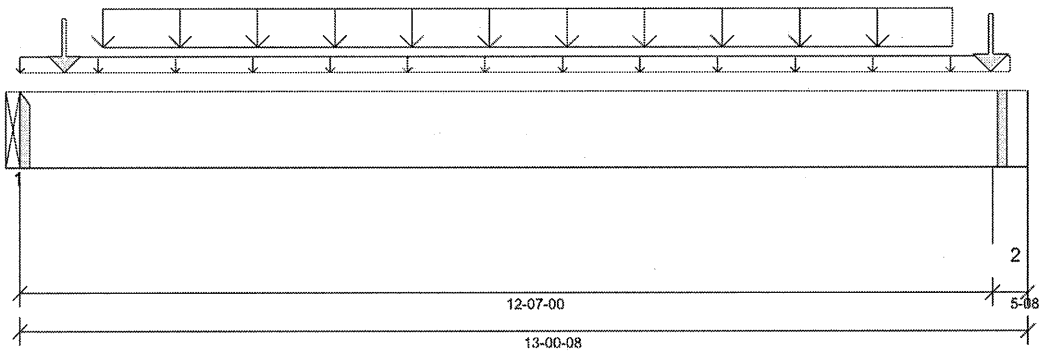
- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.

REGISTERED PROFESSIONAL ENGINEER
06/02/2022
EDWIN C. FOK
PROVINCE OF ONTARIO

S20466662

MiTek	Customer: **Gold Park Homes** Job Address: **Pine Valley Ph2** City: **Vaughan** Job Track: **45147**	Job Name: **343077 Ground A + Second A (1,** Level: **Second Floor** Label: **B6 - i50354** Type: **Beam**	**2 Ply Member** **1 3/4" x 11 7/8" 1.55E TimberStrand® LSL**	Status: **Design Passed**

Illustration Not to Scale. Pitch: 0/12 — Designed by Single Member Design Engine in MiTek® Structure Version 8.5.3.233.Update5.15 — Report Version: 2021.03.26 05/02/2022 17:08



DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)
Design Methodology: LSD
Service Condition: Dry
LL Deflection Limit: L/360,
TL Deflection Limit: L/240,

Lateral Restraint Requirements:

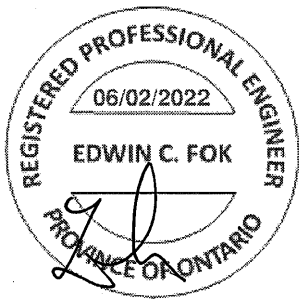
Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 0'- 9 1/2"

Factored Resistance of Support Material:

- 769 psi Beam @ 0'
- 615 psi Wall @ 12'- 8"

NAIL ONE PLY TO ANOTHER WITH 3-1/2" SPIRAL NAILS @ 9" O/C STAGGERED IN 2 ROWS



ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	6'- 6 3/4"	1.25D + 1.5L	1.00	18610 lb ft	26531 lb ft	Passed - 70%
Factored Shear:	11'- 7 1/8"	1.25D + 1.5L	1.00	6119 lb	14414 lb	Passed - 42%
Live Load (LL) Pos. Defl.:	6'- 4"	L		0.348"	L/360	Passed - L/433
Total Load (TL) Pos. Defl.:	6'- 4 1/16"	D + L		0.513"	L/240	Passed - L/294
Permanent Deflection:	6'- 4 3/16"			-	L/360	Passed - L/943

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	1-08	1.25D + 1.5L	1.00	5674 lb		6880 lb	-	Passed - 82%
2	5-08	1.25D + 1.5L	1.00	6192 lb		25225 lb	11842 lb	Passed - 52%

CONNECTOR INFORMATION

ID	Part No.	Manufacturer	Nailing Requirements			Other Information or Requirement for Reinforcement Accessories
			Top	Face	Member	
1	HGUS410		-	-	-	Connector manually specified by the user.

* Connectors: Refer to manufacturer's specifications, fasteners requirements and installation instruction. Where header fasteners are longer than the width of the supporting member, install backer block or clinch header nails.

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	13'- 1/2"	Self Weight	Top	13 lb/ft	-	-	-
Uniform	0'	12'- 9 3/4"	FC3 Floor Decking (Plan View Fill)	Top	7 lb/ft	19 lb/ft	-	-
Uniform	1'- 3/4"	12'- 3/4"	Smoothed Load	Front	189 lb/ft	424 lb/ft	-	-
Point	0'- 6 3/4"	0'- 6 3/4"	J9(i50295)	Front	157 lb	362 lb	-	-
Point	12'- 6 3/4"	12'- 6 3/4"	J9(i50177)	Front	166 lb	406 lb	-	-

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'	B7(i50432)	1272 lb	2718 lb	-	-
2	12'- 7"	13'- 1/2"	4(i41664)	1404 lb	2962 lb	-	-

DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Lateral stability factor (KL) was based on user preference to use the width of one ply.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.

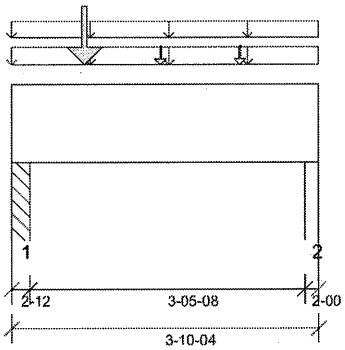
PLY TO PLY CONNECTION

- Member design assumed proper ply to ply connection by others. Fastener spacing along length of member must not exceed 4 times depth of member. Verify connection between plies according to code specification and follow the manufacturer's installation instruction. Loads assumed to be distributed equally to each ply.

SEQ46663

MiTek	Customer: **Gold Park Homes** Job Address: **Pine Valley Ph2** City: **Vaughan** Job Track: **45147**	Job Name: **343077 Ground A + Second A (1,** Level: **Second Floor** Label: **B7 - i50432** Type: **Beam**	**2 Ply Member** **1 3/4" x 11 7/8" 1.55E TimberStrand® LSL**	Status: **Design Passed**

Illustration Not to Scale. Pitch: 0/12 | Designed by Single Member Design Engine in MiTek® Structure Version 8.5.3.233.Update5.15 | Report Version: 2021.03.26 05/02/2022 17:08



DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)
Design Methodology: LSD
Service Condition: Dry
LL Deflection Limit: L/360,
TL Deflection Limit: L/240,

Lateral Restraint Requirements:

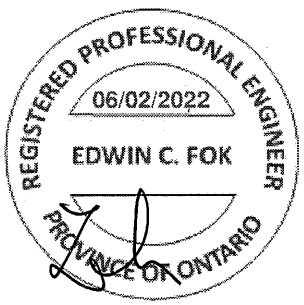
Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 0'- 10 1/2"

Factored Resistance of Support Material:

- 1334 psi Column @ 0'- 1 3/4"
- 615 psi Wall @ 3'- 9 1/4"

NAIL ONE PLY TO ANOTHER WITH 3-1/2" SPIRAL NAILS @6 " O/C STAGGERED IN 2 ROWS



ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	0'- 11"	1.25D + 1.5L	1.00	3907 lb ft	26531 lb ft	Passed - 15%
Factored Shear:	1'- 2 5/8"	1.25D + 1.5L	1.00	4966 lb	14414 lb	Passed - 34%

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	2-12	1.25D + 1.5L	1.00	5160 lb		12613 lb	12844 lb	Passed - 41%
2	2-00	1.25D + 1.5L	1.00	2136 lb		9171 lb	4306 lb	Passed - 50%

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	3'- 10 1/4"	Self Weight	Top	13 lb/ft	-	-	-
Uniform	0'	3'- 10 1/4"	User Load	Top	60 lb/ft	-	-	-
Uniform	0'	3'- 10 1/4"	FC3 Floor Decking (Plan View Fill)	Top	9 lb/ft	24 lb/ft	-	-
Point	0'- 11"	0'- 11"	B6(i50354)	Front	1272 lb	2718 lb	-	-
Point	1'- 10 1/2"	1'- 10 1/2"	J2(i50072)	Front	103 lb	254 lb	-	-
Point	2'- 10 1/2"	2'- 10 1/2"	J2(i50531)	Front	112 lb	275 lb	-	-

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 2 3/4"	Pt1(i49783)	1243 lb	2389 lb	-	-
2	3'- 8 1/4"	3'- 10 1/4"	10(i41708)	567 lb	967 lb	-	-

DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Lateral stability factor (KL) was based on user preference to use the width of one ply.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
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- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.
- User loads assume a bearing length of 3.5" in determining member capacity for loads near supports.
- Bearing capacity of member at support 1, 2 was verified for the effect of concentrated load applied near the support. At support 1. Required Load Area: L=3.500", W=3.500". LDF=1.00, Pf=5667 lb, Q'r=9555 lb, Result=59.31%.

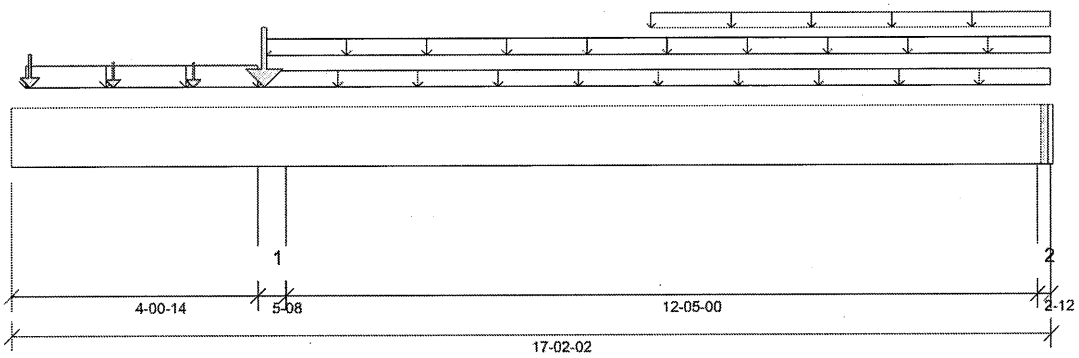
PLY TO PLY CONNECTION

- Member design assumed proper ply to ply connection by others. Fastener spacing along length of member must not exceed 4 times depth of member. Verify connection between plies according to code specification and follow the manufacturer's installation instruction. Loads assumed to be distributed equally to each ply.

SE046664

MiTek	Customer: **Gold Park Homes** Job Address: **Pine Valley Ph2** City: **Vaughan** Job Track: **45147**	Job Name: **343077 Ground A + Second A (1,** Level: **Second Floor** Label: **B8 (CANT.) - i50483** Type: **Beam**	**2 Ply Member** **1 3/4" x 11 7/8" 1.55E TimberStrand® LSL**	Status: **Design Passed**

Illustration Not to Scale. Pitch: 0/12 — Designed by Single Member Design Engine in MiTek® Structure Version 8.5.3.233.Update5.15 — Report Version: 2021.03.26 05/02/2022 17:08



DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)
Design Methodology: LSD
Service Condition: Dry
LL Deflection Limit: L/360,
TL Deflection Limit: L/240,

Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0'- 2 7/8" Bottom: 12'- 5"

Factored Resistance of Support Material:

- 615 psi Wall @ 4'- 3 5/8"
- 615 psi Wall @ 17'- 3/8"

NAIL ONE PLY TO ANOTHER WITH 3-1/2" SPIRAL NAILS @ 12in O/C STAGGERED IN 2 ROWS

ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	11'- 10 5/8"	1.25D + 1.5L	0.82	1431 lb ft	21868 lb ft	Passed - 7%
Factored Neg. Moment:	4'- 3 5/8"	1.25D + 1.5L	1.00	5448 lb ft	5612 lb ft	Passed - 97%
Factored Shear:	3'- 1"	1.25D + 1.5L	1.00	1795 lb	14414 lb	Passed - 12%
Live Load (LL) Neg. Defl.:	9'- 8 3/16"	L		0.063"	L/360	Passed - L/999
Total Load (TL) Neg. Defl.:	9'- 3 7/16"	D + L		0.069"	L/240	Passed - L/999

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	5-08	1.25D + 1.5L	1.00	4479 lb		25225 lb	11843 lb	Passed - 38%
2	2-12	1.25D + 1.5L	0.82	573 lb		10395 lb	4880 lb	Passed - 12%
2	2-12	0.9D + 1.5L	1.00		-198 lb	-	-	

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	-0'	17'- 2 1/8"	Self Weight	Top	13 lb/ft	-	-	-
Uniform	0'- 2 7/8"	4'- 7/8"	User Load	Top	38 lb/ft	100 lb/ft	-	-
Uniform	4'- 7/8"	17'- 2 1/8"	FC3 Floor Decking (Plan View Fill)	Top	8 lb/ft	21 lb/ft	-	-
Uniform	4'- 2 5/8"	17'- 2 1/8"	FC3 Floor Decking (Plan View Fill)	Top	8 lb/ft	23 lb/ft	-	-
Uniform	10'- 6 7/8"	17'- 2 1/8"	FC3 Floor Decking (Plan View Fill)	Top	5 lb/ft	-	-	-
Point	0'- 3 3/4"	0'- 3 3/4"	B5(i50292)	Front	132 lb	270 lb	-	-
Point	1'- 8 1/8"	1'- 8 1/8"	J5(i50491)	Front	61 lb	163 lb	-	-
Point	3'- 1/8"	3'- 1/8"	J5(i50024)	Front	56 lb	148 lb	-	-
Point	4'- 2 1/8"	4'- 2 1/8"	B2(i50409)	Back	386 lb	584 lb	-	-

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	4'- 7/8"	4'- 6 3/8"	10(i41708)	1129 lb	2032 lb	-	-
2	16'- 11 3/8"	17'- 2 1/8"	3(i41663)	125 lb	282/-197 lb	-	-

DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Lateral stability factor (KL) was based on user preference to use the width of one ply.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- The deflection at the cantilever for either live and/or total loads is less than 3/8" and therefore has been excluded from the deflection ratio considerations.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.

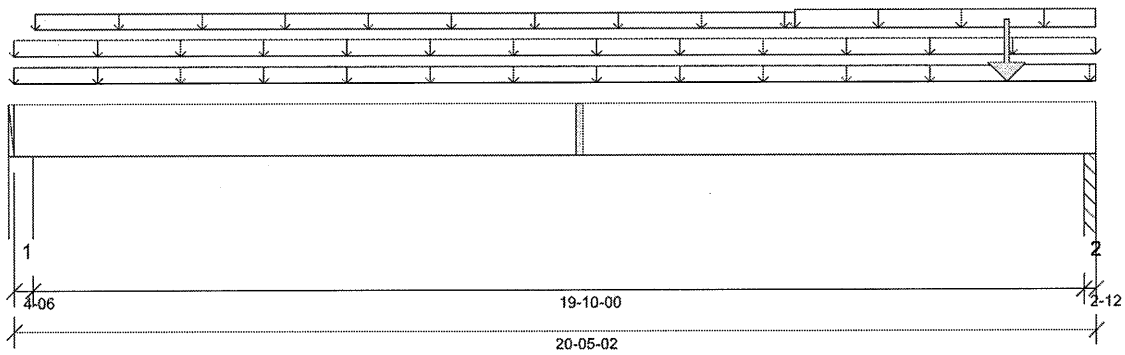
PLY TO PLY CONNECTION

- Member design assumed proper ply to ply connection by others. Fastener spacing along length of member must not exceed 4 times depth of member. Verify connection between plies according to code specification and follow the manufacturer's installation instruction. Loads assumed to be distributed equally to each ply.

S60466665

MiTek	Customer: **Gold Park Homes** Job Address: **Pine Valley Ph2** City: **Vaughan** Job Track: **45147**	Job Name: **343077 Ground A + Second A (1** Level: **Second Floor** Label: **B9 - i49806** Type: **Beam**	**2 Ply Member** **1 3/4" x 11 7/8" 1.55E TimberStrand® LSL**	Status: **Design Passed**

Illustration Not to Scale. Pitch: 0/12 — Designed by Single Member Design Engine in MiTek® Structure Version 8.5.3.233.Update5.15 — Report Version: 2021.03.26 05/02/2022 17:09



DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)
Design Methodology: LSD
Service Condition: Dry
LL Deflection Limit: L/360,
TL Deflection Limit: L/240,

Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 10'- 3 3/4"

Factored Resistance of Support Material:

- 615 psi Wall @ 0'- 3 3/8"
- 1334 psi Column @ 20'- 3 3/8"

NAIL ONE PLY TO ANOTHER WITH 3-1/2" SPIRAL NAILS @ 12" O/C STAGGERED IN 2 ROWS

REGISTERED PROFESSIONAL ENGINEER
06/02/2022
EDWIN C. FOK
PROVINCE OF ONTARIO

ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	15'- 8 13/16"	1.25D + 1.5L	1.00	9437 lb ft	26531 lb ft	Passed - 36%
Factored Shear:	19'- 2 1/2"	1.25D + 1.5L	1.00	5658 lb	14414 lb	Passed - 39%
Live Load (LL) Pos. Defl.:	11'- 2 7/8"	L		0.355"	L/360	Passed - L/670
Total Load (TL) Pos. Defl.:	11'- 1 7/8"	D + L		0.631"	L/240	Passed - L/377
Permanent Deflection:	11'- 9/16"			-	L/360	Passed - L/888

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	4-06	1.25D + 1.5L	1.00	1179 lb		20065 lb	9420 lb	Passed - 13%
2	2-12	1.25D + 1.5L	1.00	5861 lb		12613 lb	12844 lb	Passed - 46%

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	20'- 5 1/8"	Self Weight	Top	13 lb/ft	-	-	-
Uniform	0'	20'- 5 1/8"	FC3 Floor Decking (Plan View Fill)	Top	5 lb/ft	14 lb/ft	-	-
Uniform	0'	18'- 8 7/8"	FC3 Floor Decking (Plan View Fill)	Top	4 lb/ft	11 lb/ft	-	-
Uniform	0'- 4 7/8"	14'- 8 7/8"	FC3 Floor Decking (Plan View Fill)	Top	3 lb/ft	-	-	-
Uniform	14'- 8 7/8"	20'- 5 1/8"	User Load	Top	60 lb/ft	-	-	-
Uniform	18'- 8 7/8"	20'- 5 1/8"	FC3 Floor Decking (Plan View Fill)	Top	-	24 lb/ft	-	-
Point	18'- 8 7/8"	18'- 8 7/8"	B4(i49924)	Back	1107 lb	2514 lb	-	-

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 4 3/8"	E12(i41614)	393 lb	458 lb	-	-
2	20'- 2 3/8"	20'- 5 1/8"	Pt1(i49783)	1570 lb	2599 lb	-	-

DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Lateral stability factor (KL) was based on user preference to use the width of one ply.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.

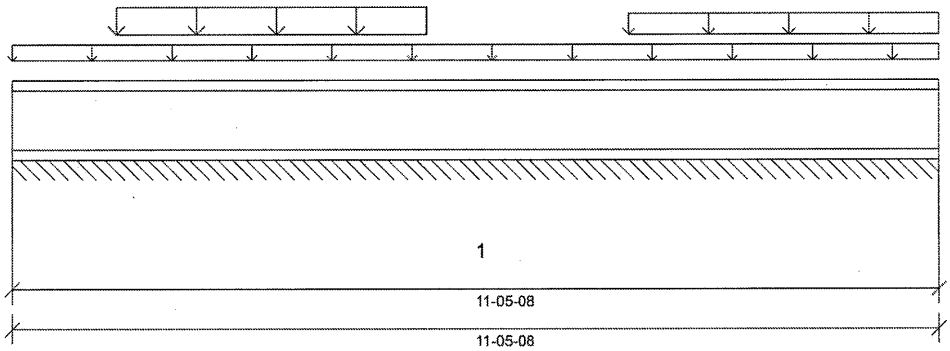
PLY TO PLY CONNECTION

- Member design assumed proper ply to ply connection by others. Fastener spacing along length of member must not exceed 4 times depth of member. Verify connection between plies according to code specification and follow the manufacturer's installation instruction. Loads assumed to be distributed equally to each ply.

SE046666

MiTek	Customer: **Gold Park Homes** Job Address: **Pine Valley Ph2** City: **Vaughan** Job Track: **45147**	Job Name: **343077 Ground A + Second A (1** Level: **Second Floor** Label: **B10 (-7R) - i49990** Type: **Beam**	**1 Ply Member** **11 7/8" NI-20**	Status: **Design Passed**

Illustration Not to Scale. Pitch: 0/12 | Designed by Single Member Design Engine in MiTek® Structure Version 8.5.3.233.Update5.15 | Report Version: 2021.03.26 05/02/2022 17:09



DESIGN INFORMATION

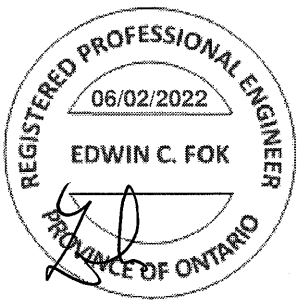
Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)
Design Methodology: LSD
Service Condition: Dry
LL Deflection Limit: ,
TL Deflection Limit: ,

Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 0'

Factored Resistance of Support Material:



SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
	N/A	1.25D + 1.5L	1.00	396 lb/ft		3300 lb/ft	-	Passed - 12%
	N/A	1.4D	0.65	59 lb/ft		2145 lb/ft	-	Passed - 3%

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	11'- 5 1/2"	Self Weight	Top	3 lb/ft	-	-	-
Uniform	0'	11'- 5 1/2"	FC4 Floor Decking (Plan View Fill)	Top	6 lb/ft	15 lb/ft	-	-
Uniform	1'- 3 1/2"	5'- 1 1/2"	User Load	Top	69 lb/ft	184 lb/ft	-	-
Uniform	7'- 7 1/2"	11'- 5 1/2"	FC4 Floor Decking (Plan View Fill)	Top	34 lb/ft	89 lb/ft	-	-

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	11'- 5 1/2"	-	189 lb	487 lb	-	-
++>	0'	1'- 9 1/2"	2(i41666)	8 lb/ft	15 lb/ft	-	-
++>	1'- 3 1/2"	1'- 9 1/2"	2(i41666)	69 lb/ft	184 lb/ft	-	-
++>	1'- 9 1/2"	5'- 1 1/2"	5(i41694)	69 lb/ft	184 lb/ft	-	-
++>	1'- 9 1/2"	11'- 5 1/2"	5(i41694)	8 lb/ft	15 lb/ft	-	-
++>	7'- 7 1/2"	11'- 5 1/2"	5(i41694)	34 lb/ft	89 lb/ft	-	-

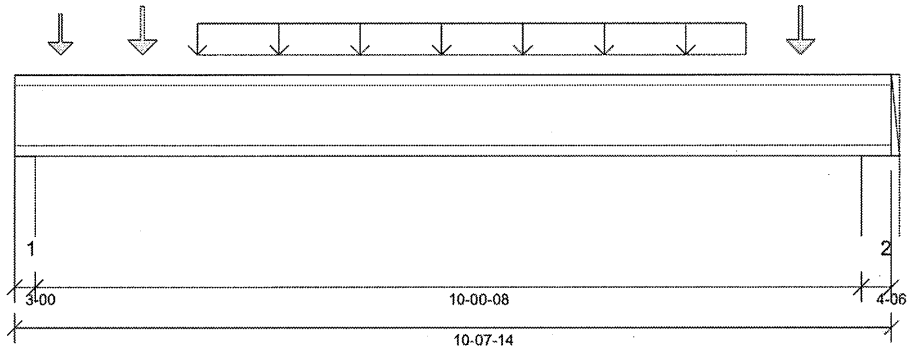
DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.

83046667

MiTek	Customer: **Gold Park Homes** Job Address: **Pine Valley Ph2** City: **Vaughan** Job Track: **45147**	Job Name: **343077 Ground A + Second A (1** Level: **Second Floor** Label: **B11 (-7R) - i49912** Type: **Beam**	**2 Ply Member** **11 7/8" NI-20**	Status: **Design Passed**

Illustration Not to Scale. Pitch: 0/12 | Designed by Single Member Design Engine in MiTek® Structure Version 8.5.3.233.Update5.15 | Report Version: 2021.03.26 05/02/2022 17:10



DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)
Design Methodology: LSD
Service Condition: Dry
LL Deflection Limit: L/360,
TL Deflection Limit: L/240,

Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 1'- 1 1/2"

Factored Resistance of Support Material:

- 615 psi Wall @ 0'- 2"
- 615 psi Wall @ 10'- 4 1/2"

ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	5'- 6 5/8"	1.25D + 1.5L	1.00	5996 lb ft	11160 lb ft	Passed - 54%
Factored Shear:	0'- 3 1/16"	1.25D + 1.5L	1.00	2350 lb	4480 lb	Passed - 52%
Live Load (LL) Pos. Defl.:	5'- 3 1/4"	L		0.133"	L/360	Passed - L/903
Total Load (TL) Pos. Defl.:	5'- 3 1/4"	D + L		0.187"	L/240	Passed - L/645

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	3-00	1.25D + 1.5L	1.00	2352 lb		4240 lb	9228 lb	Passed - 55%
2	4-06	1.25D + 1.5L	1.00	2198 lb		4480 lb	13457 lb	Passed - 49%

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	10'- 7 7/8"	Self Weight	Top	6 lb/ft	-	-	-
Uniform	2'- 2 5/8"	8'- 10 5/8"	Smoothed Load	Front	86 lb/ft	230 lb/ft	-	-
Point	0'- 6 5/8"	0'- 6 5/8"	J3(i50278)	Front	78 lb	208 lb	-	-
Point	1'- 6 5/8"	1'- 6 5/8"	J3(i50478)	Front	100 lb	267 lb	-	-
Point	9'- 6 5/8"	9'- 6 5/8"	J3(i50507)	Front	100 lb	267 lb	-	-

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 3"	2(i41666)	471 lb	1175 lb	-	-
2	10'- 3 1/2"	10'- 7 7/8"	E24(i41700)	442 lb	1097 lb	-	-

DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.

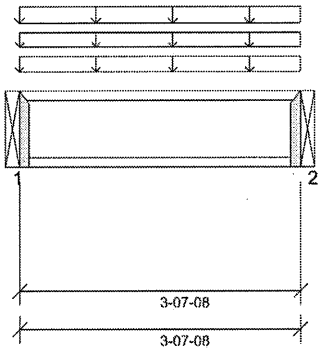
PLY TO PLY CONNECTION

- Member design assumed proper ply to ply connection by others. Verify connection between plies according to code specification and follow the manufacturer's installation instruction. Loads assumed to be distributed equally to each ply.

SEQ46668

MiTek	Customer: **Gold Park Homes** Job Address: **Pine Valley Ph2** City: **Vaughan** Job Track: **45147**	Job Name: **343077 Ground A + Second A (1,** Level: **Ground Floor** Label: **B12 - i50356** Type: **Beam**	**1 Ply Member** **11 7/8" NI-20**	Status: **Design Passed**

Illustration Not to Scale. Pitch: 0/12 | Designed by Single Member Design Engine in MiTek® Structure Version 8.5.3.233.Update5.15 | Report Version: 2021.03.26 05/02/2022 17:10



DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)
Design Methodology: LSD
Service Condition: Dry
LL Deflection Limit: L/360,
TL Deflection Limit: L/240,

Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 3'- 7 1/2"

Factored Resistance of Support Material:

- 769 psi Beam @ 0'
- 769 psi Beam @ 3'- 7 1/2"

ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	1'- 9 3/4"	1.25D + 1.5L	1.00	164 lb ft	5580 lb ft	Passed - 3%
Factored Shear:	3'- 7 7/16"	1.25D + 1.5L	1.00	181 lb	2240 lb	Passed - 8%

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	1-12	1.25D + 1.5L	1.00	187 lb		1970 lb	-	Passed - 10%
2	1-12	1.25D + 1.5L	1.00	181 lb		1970 lb	-	Passed - 9%

CONNECTOR INFORMATION

ID	Part No.	Manufacturer	Nailing Requirements			Other Information or Requirement for Reinforcement Accessories
			Top	Face	Member	
1	LT251188		-	-	-	Connector manually specified by the user.
2	LT251188		-	-	-	Connector manually specified by the user.

* Connectors: Refer to manufacturer's specifications, fasteners requirements and installation instruction. Where header fasteners are longer than the width of the supporting member, install backer block or clinch header nails.

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	3'- 7 1/2"	Self Weight	Top	3 lb/ft	-	-	-
Uniform	0'	3'- 7 1/2"	User Load	Top	13 lb/ft	34 lb/ft	-	-
Uniform	0'	3'- 7 1/2"	FC2 Floor Decking (Plan View Fill)	Top	5 lb/ft	14 lb/ft	-	-
Uniform	0'	3'- 7 1/2"	FC2 Floor Decking (Plan View Fill)	Top	2 lb/ft	-	-	-

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'	B15(i50756)	43 lb	89 lb	-	-
2	3'- 7 1/2"	3'- 7 1/2"	B13(i50410)	41 lb	87 lb	-	-

DESIGN NOTES

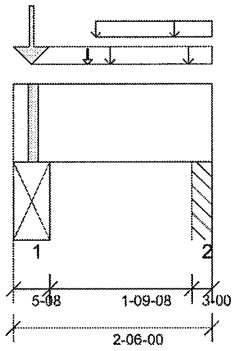
- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.

REGISTERED PROFESSIONAL ENGINEER
06/02/2022
EDWIN C. FOK
PROVINCE OF ONTARIO

S6046669

MiTek	Customer: **Gold Park Homes** Job Address: **Pine Valley Ph2** City: **Vaughan** Job Track: **45147**	Job Name: **343077 Ground A + Second A (1** Level: **Ground Floor** Label: **B13 - i50410** Type: **Beam**	**2 Ply Member** **1 3/4" x 11 7/8" 1.55E TimberStrand® LSL**	Status: **Design Passed**

Illustration Not to Scale. Pitch: 0/12 | Designed by Single Member Design Engine in MiTek® Structure Version 8.5.3.233.Update5.15 | Report Version: 2021.03.26 05/02/2022 17:10



DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)
Design Methodology: LSD
Service Condition: Dry
LL Deflection Limit: L/360,
TL Deflection Limit: L/240,

Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 1'- 5 1/2"

Factored Resistance of Support Material:

- 769 psi Beam @ 0'- 4 1/2"

NAIL ONE PLY TO ANOTHER WITH 3-1/2" SPIRAL NAILS @ 4" O/C STAGGERED IN 2 ROWS

REGISTERED PROFESSIONAL ENGINEER
06/02/2022
EDWIN C. FOK
PROVINCE OF ONTARIO

ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Neg. Moment:	0'- 4 1/2"	1.25D + 1.5L	1.00	1146 lb ft	25307 lb ft	Passed - 5%
Factored Moment:	0'- 4 1/2"	1.25D + 1.5L	1.00	1146 lb ft	25307 lb ft	Passed - 5%
Factored Moment:				0 lb ft	0 lb ft	
Factored Moment:				0 lb ft	0 lb ft	
Factored Shear:	1'- 5 3/8"	1.25D + 1.5L	1.00	704 lb	14414 lb	Passed - 5%
Live Load (LL) Deflection:	1'- 2 5/16"	L		0.000"	L/360	Passed - L/999
Total Load (TL) Deflection:	1'- 2 1/8"	D + L		0.000"	L/240	Passed - L/999

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	5-08	1.25D + 1.5L	1.00	8689 lb		25225 lb	14803 lb	Passed - 59%
2	3-00	1.25D + 1.5L	1.00		-361 lb	-	-	

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	2'- 6"	Self Weight	Top	13 lb/ft	-	-	-
Uniform	0'- 2 3/4"	2'- 6"	User Load	Top	60 lb/ft	-	-	-
Uniform	1'- 1/2"	2'- 6"	FC2 Floor Decking (Plan View Fill)	Top	-	26 lb/ft	-	-
Point	0'- 11 1/4"	0'- 11 1/4"	B12(i50356)	Back	41 lb	87 lb	-	-
Point	0'- 2 3/4"	0'- 2 3/4"	Pt1(i49783)	Top	2042 lb	3523 lb	-	-

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 5 1/2"	ST. BEAM (DR.)(i41671)	2550 lb	4272 lb	-	-
2	2'- 3"	2'- 6"	Pt1(i49863)	-280 lb	-612 lb	-	-

DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Lateral stability factor (KL) was based on user preference to use the width of one ply.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.
- Bearing capacity of member at support 1, 2 was verified for the effect of concentrated load applied near the support. At support 1. Required Load Area: L=5.500", W=3.499". LDF=1.00, Pf=7837 lb, Q'r=16814 lb, Result=46.61%.

PLY TO PLY CONNECTION

- Member design assumed proper ply to ply connection by others. Fastener spacing along length of member must not exceed 4 times depth of member. Verify connection between plies according to code specification and follow the manufacturer's installation instruction. Loads assumed to be distributed equally to each ply.

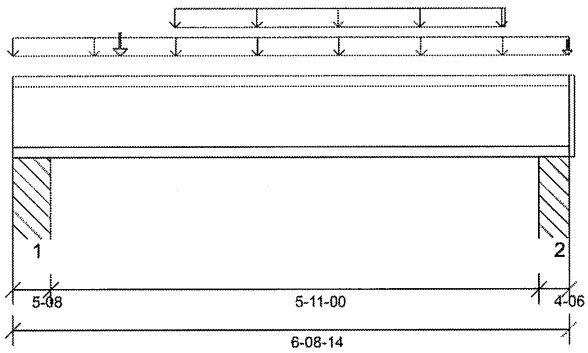
S3046670

MiTek	Customer: **Gold Park Homes** Job Address: **Pine Valley Ph2** City: **Vaughan** Job Track: **45147**	Job Name: **343077 Ground A + Second A (1,** Level: **Ground Floor** Label: **B14 - i49988** Type: **Beam**	**1 Ply Member** **11 7/8" NI-20**	Status: **Design Passed**

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in MiTek® Structure Version 8.5.3.233.Update5.15

Report Version: 2021.03.26 05/02/2022 17:10



DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)
Design Methodology: LSD
Service Condition: Dry
LL Deflection Limit: L/360,
TL Deflection Limit: L/240,

Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 1'- 1 1/2"

Factored Resistance of Support Material:

- 1334 psi Column @ 0'- 4 1/2"
- 1334 psi Column @ 6'- 5 1/2"

REGISTERED PROFESSIONAL ENGINEER
06/02/2022
EDWIN C. FOK
PROVINCE OF ONTARIO

ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	3'- 6 3/4"	1.25D + 1.5L	0.86	814 lb ft	4780 lb ft	Passed - 17%
Factored Shear:	0'- 5 9/16"	1.25D + 1.5L	0.86	504 lb	1919 lb	Passed - 26%
Live Load (LL) Pos. Defl.:	3'- 5"	L		0.010"	L/360	Passed - L/999
Total Load (TL) Pos. Defl.:	3'- 5"	D + L		0.025"	L/240	Passed - L/999

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	5-08	1.25D + 1.5L	0.86	540 lb		1919 lb	15718 lb	Passed - 28%
2	4-06	1.25D + 1.5L	0.86	527 lb		1919 lb	12503 lb	Passed - 27%

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	6'- 8 7/8"	Self Weight	Top	3 lb/ft	-	-	-
Uniform	0'	6'- 8 7/8"	User Load	Top	60 lb/ft	-	-	-
Uniform	1'- 11 5/8"	5'- 11 5/8"	Smoothed Load	Front	20 lb/ft	51 lb/ft	-	-
Point	1'- 3 5/8"	1'- 3 5/8"	J6(i50551)	Front	24 lb	64 lb	-	-
Point	6'- 8 5/8"	6'- 8 5/8"	FC2 Floor Decking (Plan View Fill)	Top	-	5 lb	-	-

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 5 1/2"	Pt1(i49863)	267 lb	138 lb	-	-
2	6'- 4 1/2"	6'- 8 7/8"	Pt1(i49963)	260 lb	135 lb	-	-

DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.

SE04667

Customer: **Gold Park Homes** Job Address: **Pine Valley Ph2** City: **Vaughan** Job Track: **45147**	Job Name: **343077 Ground A + Second A (1,** Level: **Ground Floor** Label: **B15 - i50756** Type: **Beam**	**1 Ply Member** **11 7/8" NI-20**	Status: **Design Passed**

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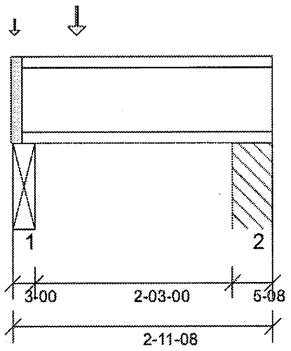


DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)
Design Methodology: LSD
Service Condition: Dry
LL Deflection Limit: L/360,
TL Deflection Limit: L/240,

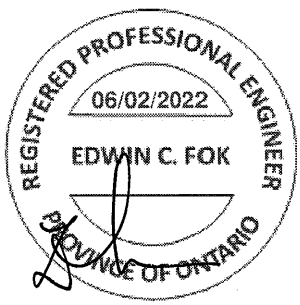
Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 2'- 1 1/2" Bottom: 2'- 1 1/2"

Factored Resistance of Support Material:

- 769 psi Beam @ 0'- 2"
- 1334 psi Column @ 2'- 7"



ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	0'- 8 3/4"	1.25D + 1.5L	1.00	82 lb ft	5580 lb ft	Passed - 1%
Factored Shear:	0'- 3 1/16"	1.25D + 1.5L	1.00	148 lb	2240 lb	Passed - 7%

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	3-00	1.25D + 1.5L	1.00	151 lb		2120 lb	5770 lb	Passed - 7%
2	5-08	1.25D + 1.5L	1.00	49 lb		2240 lb	18348 lb	Passed - 2%

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	2'- 11 1/2"	Self Weight	Top	3 lb/ft	-	-	-
Point	0'- 8 3/4"	0'- 8 3/4"	B12(i50356)	Front	43 lb	89 lb	-	-
Point	0'- 1/4"	0'- 1/4"	FC2 Floor Decking (Plan View Fill)	Top	1 lb	1 lb	-	-

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 3"	ST. BEAM (DR.)(i41671)	38 lb	70 lb	-	-
2	2'- 6"	2'- 11 1/2"	Pt3(i49948)	15 lb	21 lb	-	-

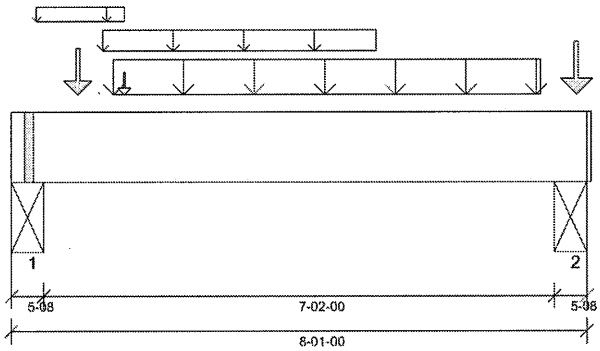
DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.

SE046672

MiTek	Customer: **Gold Park Homes** Job Address: **Pine Valley Ph2** City: **Vaughan** Job Track: **45147**	Job Name: **343077 Ground A + Second A (1,** Level: **Ground Floor** Label: **B16 - i49929** Type: **Beam**	**1 Ply Member** **1 3/4" x 11 7/8" 1.55E TimberStrand® LSL**	Status: **Design Passed**

Illustration Not to Scale. Pitch: 0/12 | Designed by Single Member Design Engine in MiTek® Structure Version 8.5.3.233.Update5.15 | Report Version: 2021.03.26 05/02/2022 17:11



DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)
Design Methodology: LSD
Service Condition: Dry
LL Deflection Limit: L/360,
TL Deflection Limit: L/240,

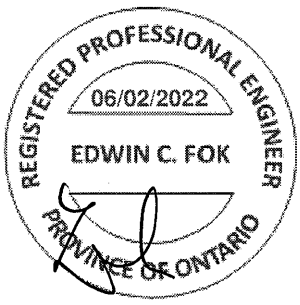
Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 0'- 9 1/2"

Factored Resistance of Support Material:

- 769 psi Beam @ 0'- 4 1/2"
- 769 psi Beam @ 7'- 8 1/2"



ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	3'- 11 1/4"	1.25D + 1.5L	1.00	8725 lb ft	13266 lb ft	Passed - 66%
Factored Neg. Moment:	7'- 8 1/2"	1.25D + 1.5L	1.00	215 lb ft	13266 lb ft	Passed - 2%
Factored Shear:	1'- 5 3/8"	1.25D + 1.5L	1.00	4425 lb	7207 lb	Passed - 61%
Live Load (LL) Pos. Defl.:	3'- 11 7/8"	L		0.096"	L/360	Passed - L/896
Total Load (TL) Pos. Defl.:	4'	D + L		0.161"	L/240	Passed - L/533

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	5-08	1.25D + 1.5L	1.00	4603 lb		12613 lb	7402 lb	Passed - 62%
2	5-08	1.25D + 1.5L	1.00	5001 lb		12613 lb	7402 lb	Passed - 68%

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	8'- 1"	Self Weight	Top	6 lb/ft	-	-	-
Uniform	0'- 4 1/4"	1'- 7 1/8"	FC2 Floor Decking (Plan View Fill)	Top	-	45 lb/ft	-	-
Uniform	1'- 3 1/2"	5'- 1 1/2"	User Load	Top	69 lb/ft	184 lb/ft	-	-
Uniform	1'- 5 1/4"	7'- 5 1/4"	Smoothed Load	Front	315 lb/ft	408 lb/ft	-	-
Point	0'- 11 1/4"	0'- 11 1/4"	J8(i50720)	Front	245 lb	318 lb	-	-
Point	7'- 11 1/4"	7'- 11 1/4"	J8(i50727)	Front	261 lb	407 lb	-	-
Point	1'- 7 1/8"	1'- 7 1/8"	J2(i50533)	Back	42 lb	113 lb	-	-

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 5 1/2"	ST. BEAM (DR.)(i41668)	1322 lb	1990 lb	-	-
2	7'- 7 1/2"	8'- 1"	ST. BEAM (DR.)(i41671)	1466 lb	2088 lb	-	-

DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.

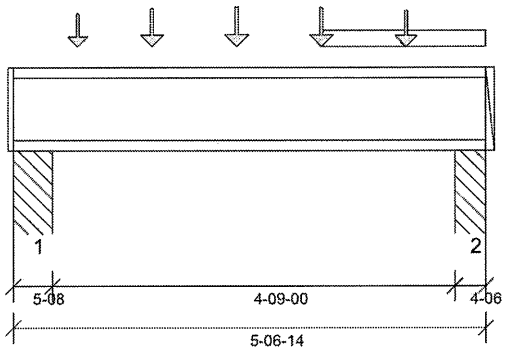
SE046673

MiTek	Customer: **Gold Park Homes** Job Address: **Pine Valley Ph2** City: **Vaughan** Job Track: **45147**	Job Name: **343077 Ground A + Second A (1** Level: **Ground Floor** Label: **B17 (-3R) - i49970** Type: **Beam**	**1 Ply Member** **11 7/8" NI-20**	Status: **Design Passed**

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in MiTek® Structure Version 8.5.3.233.Update5.15

Report Version: 2021.03.26 05/02/2022 17:11



DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)
Design Methodology: LSD
Service Condition: Dry
LL Deflection Limit: L/360,
TL Deflection Limit: L/240,

Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 0'- 9 1/2"

Factored Resistance of Support Material:

- 1334 psi Column @ 0'- 4 1/2"
- 1334 psi Column @ 5'- 3 1/2"

REGISTERED PROFESSIONAL ENGINEER
06/02/2022
EDWIN C. FOK
PROVINCE OF ONTARIO

ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	2'- 7 5/8"	1.25D + 1.5L	1.00	1118 lb ft	5580 lb ft	Passed - 20%
Factored Shear:	0'- 5 9/16"	1.25D + 1.5L	1.00	869 lb	2240 lb	Passed - 39%
Live Load (LL) Pos. Defl.:	2'- 10"	L		0.017"	L/360	Passed - L/999
Total Load (TL) Pos. Defl.:	2'- 10"	D + L		0.026"	L/240	Passed - L/999

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	5-08	1.25D + 1.5L	1.00	880 lb		2240 lb	18348 lb	Passed - 39%
2	4-06	1.25D + 1.5L	1.00	826 lb		2240 lb	14595 lb	Passed - 37%

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	5'- 6 7/8"	Self Weight	Top	3 lb/ft	-	-	-
Tapered	3'- 7 5/8"	5'- 6 7/8"	FC1 Floor Decking (Plan View Fill)	Top	4 To 3 lb/ft	11 To 8 lb/ft	-	-
Point	0'- 9 1/8"	0'- 9 1/8"	J3(i49975)	Back	62 lb	123 lb	-	-
Point	1'- 7 5/8"	1'- 7 5/8"	J3(i50443)	Back	80 lb	161 lb	-	-
Point	2'- 7 5/8"	2'- 7 5/8"	J3(i49874)	Back	86 lb	172 lb	-	-
Point	3'- 7 5/8"	3'- 7 5/8"	J3(i50377)	Back	83 lb	166 lb	-	-
Point	4'- 7 5/8"	4'- 7 5/8"	J3(i50332)	Back	74 lb	148 lb	-	-

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 5 1/2"	Pt2(i49962)	213 lb	409 lb	-	-
2	5'- 2 1/2"	5'- 6 7/8"	Pt2(i49898)	200 lb	384 lb	-	-

DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.

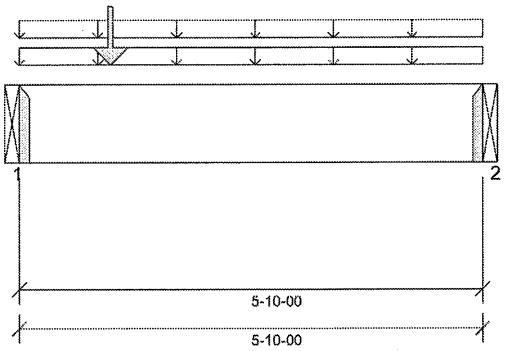
SE046674

MiTek	Customer: **Gold Park Homes** Job Address: **Pine Valley Ph2** City: **Vaughan** Job Track: **45147**	Job Name: **343077 Ground A + Second A (1,** Level: **Ground Floor** Label: **B18 (-3R) - i49844** Type: **Beam**	**2 Ply Member** **1 3/4" x 11 7/8" 1.55E TimberStrand® LSL**	Status: **Design Passed**

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in MiTek® Structure Version 8.5.3.233.Update5.15

Report Version: 2021.03.26 05/02/2022 17:12



DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)
Design Methodology: LSD
Service Condition: Dry
LL Deflection Limit: L/360,
TL Deflection Limit: L/240,

Lateral Restraint Requirements:

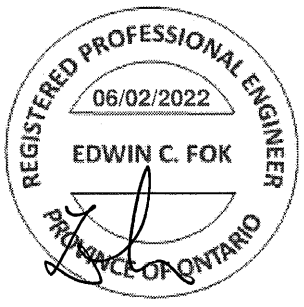
Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 5'- 10"

Factored Resistance of Support Material:

- 769 psi Beam @ 0'
- 769 psi Beam @ 5'- 10"

NAIL ONE PLY TO ANOTHER WITH 3-1/2" SPIRAL NAILS @ 6 " O/C STAGGERED IN 2 ROWS



ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	1'- 1 3/4"	1.25D + 1.5L	1.00	6609 lb ft	26531 lb ft	Passed - 25%
Factored Shear:	0'- 11 7/8"	1.25D + 1.5L	1.00	5694 lb	14414 lb	Passed - 40%
Live Load (LL) Pos. Defl.:	2'- 6 3/4"	L		0.018"	L/360	Passed - L/999
Total Load (TL) Pos. Defl.:	2'- 7 1/16"	D + L		0.031"	L/240	Passed - L/999

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	1-08	1.25D + 1.5L	1.00	5891 lb		6880 lb	-	Passed - 86%
2	1-08	1.25D + 1.5L	1.00	1844 lb		6880 lb	-	Passed - 27%

CONNECTOR INFORMATION

ID	Part No.	Manufacturer	Nailing Requirements			Other Information or Requirement for Reinforcement Accessories
			Top	Face	Member	
1	HGUS410		-	-	-	Connector manually specified by the user.
2	HGUS410		-	-	-	Connector manually specified by the user.

* Connectors: Refer to manufacturer's specifications, fasteners requirements and installation instruction. Where header fasteners are longer than the width of the supporting member, install backer block or clinch header nails.

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	5'- 10"	Self Weight	Top	13 lb/ft	-	-	-
Uniform	0'	5'- 10"	User Load	Top	60 lb/ft	-	-	-
Uniform	0'	5'- 10"	FC1 Floor Decking (Plan View Fill)	Top	16 lb/ft	42 lb/ft	-	-
Point	1'- 1 3/4"	1'- 1 3/4"	10(i41708)	Top	1734 lb	2999 lb	-	-

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'	B19 (-3R)(i49909)	1660 lb	2544 lb	-	-
2	5'- 10"	5'- 10"	B20 (-3R)(i50485)	607 lb	724 lb	-	-

DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Lateral stability factor (KL) was based on user preference to use the width of one ply.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.

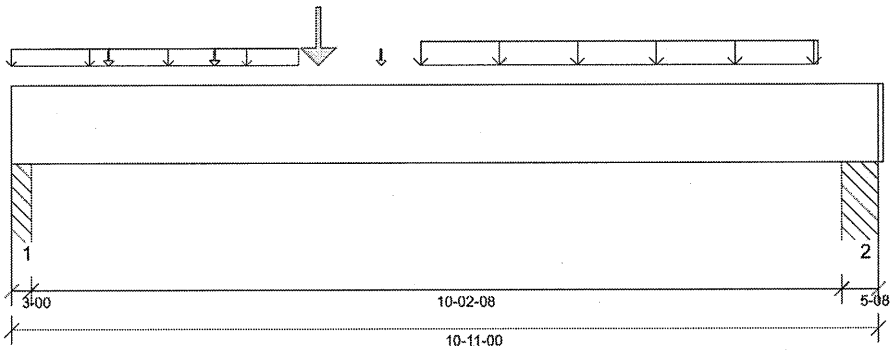
PLY TO PLY CONNECTION

- Member design assumed proper ply to ply connection by others. Fastener spacing along length of member must not exceed 4 times depth of member. Verify connection between plies according to code specification and follow the manufacturer's installation instruction. Loads assumed to be distributed equally to each ply.

S6046675

MiTek	Customer: **Gold Park Homes** Job Address: **Pine Valley Ph2** City: **Vaughan** Job Track: **45147**	Job Name: **343077 Ground A + Second A (1,** Level: **Ground Floor** Label: **B19 (-3R) - i49909** Type: **Beam**	**2 Ply Member** **1 3/4" x 11 7/8" 1.55E** **TimberStrand® LSL**	Status: **Design Passed**

Illustration Not to Scale. Pitch: 0/12 | Designed by Single Member Design Engine in MiTek® Structure Version 8.5.3.233.Update5.15 | Report Version: 2021.03.26 05/02/2022 17:12



DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)
Design Methodology: LSD
Service Condition: Dry
LL Deflection Limit: L/360,
TL Deflection Limit: L/240,

Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 1'- 1 1/2"

Factored Resistance of Support Material:

- 1334 psi Column @ 0'- 2"
- 1334 psi Column @ 10'- 6 1/2"

NAIL ONE PLY TO ANOTHER WITH 3-1/2" SPIRAL NAILS @ 12" O/C STAGGERED IN 2 ROWS

REGISTERED PROFESSIONAL ENGINEER
06/02/2022
EDWIN C. FOK
PROVINCE OF ONTARIO

ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	3'- 10 1/2"	1.25D + 1.5L	1.00	17160 lb ft	26531 lb ft	Passed - 65%
Factored Shear:	1'- 2 7/8"	1.25D + 1.5L	1.00	5062 lb	14414 lb	Passed - 35%
Live Load (LL) Pos. Defl.:	5'- 9/16"	L		0.166"	L/360	Passed - L/735
Total Load (TL) Pos. Defl.:	5'- 9/16"	D + L		0.272"	L/240	Passed - L/450

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	3-00	1.25D + 1.5L	1.00	5191 lb		13759 lb	14011 lb	Passed - 38%
2	5-08	1.25D + 1.5L	1.00	3484 lb		25225 lb	25687 lb	Passed - 14%

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	10'- 11"	Self Weight	Top	13 lb/ft	-	-	-
Uniform	0'	3'- 7 1/2"	User Load	Top	13 lb/ft	34 lb/ft	-	-
Uniform	5'- 1 15/16"	10'- 1 15/16"	Smoothed Load	Back	63 lb/ft	127 lb/ft	-	-
Point	1'- 2 3/4"	1'- 2 3/4"	J4(i50290)	Back	67 lb	178 lb	-	-
Point	2'- 6 3/4"	2'- 6 3/4"	J4(i50382)	Back	64 lb	170 lb	-	-
Point	3'- 10 1/2"	3'- 10 1/2"	B18 (-3R)(i49844)	Back	1660 lb	2544 lb	-	-
Point	4'- 7 15/16"	4'- 7 15/16"	J4(i50516)	Back	57 lb	115 lb	-	-

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 3"	Pt2(i50280)	1416 lb	2281 lb	-	-
2	10'- 5 1/2"	10'- 11"	Pt2(i49934)	961 lb	1522 lb	-	-

DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Lateral stability factor (KL) was based on user preference to use the width of one ply.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.

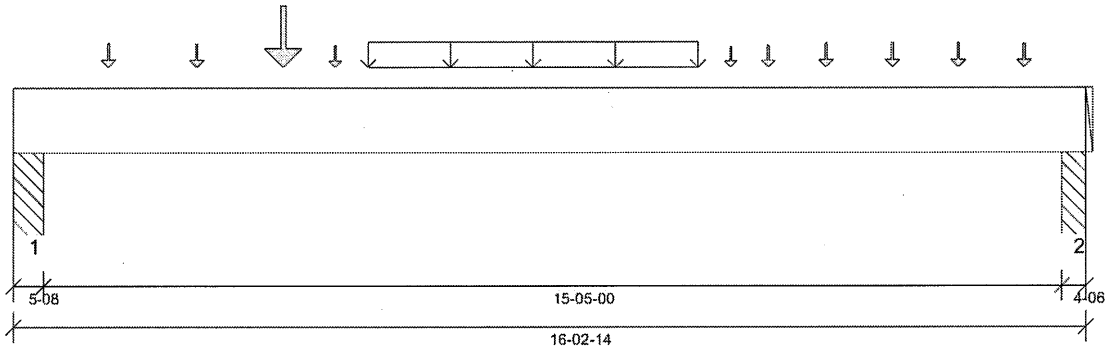
PLY TO PLY CONNECTION

- Member design assumed proper ply to ply connection by others. Fastener spacing along length of member must not exceed 4 times depth of member. Verify connection between plies according to code specification and follow the manufacturer's installation instruction. Loads assumed to be distributed equally to each ply.

E3046676

MiTek	Customer: **Gold Park Homes** Job Address: **Pine Valley Ph2** City: **Vaughan** Job Track: **45147**	Job Name: **343077 Ground A + Second A (1,** Level: **Ground Floor** Label: **B20 (-3R) - i50485** Type: **Beam**	**2 Ply Member** **1 3/4" x 11 7/8" 1.55E TimberStrand® LSL**	Status: **Design Passed**

Illustration Not to Scale. Pitch: 0/12 | Designed by Single Member Design Engine in MiTek® Structure Version 8.5.3.233.Update5.15 | Report Version: 2021.03.26 05/02/2022 17:12



DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)
Design Methodology: LSD
Service Condition: Dry
LL Deflection Limit: L/360,
TL Deflection Limit: L/240,

Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 1'- 1 1/2"

Factored Resistance of Support Material:

- 1334 psi Column @ 0'- 4 1/2"
- 1334 psi Column @ 15'- 11 1/2"

NAIL ONE PLY TO ANOTHER WITH 3-1/2" SPIRAL NAILS @12" O/C STAGGERED IN 2 ROWS

REGISTERED PROFESSIONAL ENGINEER
06/02/2022
EDWIN C. FOK
PROVINCE OF ONTARIO

ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	6'- 10 7/16"	1.25D + 1.5L	1.00	12333 lb ft	26531 lb ft	Passed - 46%
Factored Shear:	1'- 5 3/8"	1.25D + 1.5L	1.00	3348 lb	14414 lb	Passed - 23%
Live Load (LL) Pos. Defl.:	8'- 1/8"	L		0.322"	L/360	Passed - L/574
Total Load (TL) Pos. Defl.:	7'- 11 11/16"	D + L		0.533"	L/240	Passed - L/347
Permanent Deflection:	7'- 11 1/8"			-	L/360	Passed - L/903

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	5-08	1.25D + 1.5L	1.00	3371 lb		25225 lb	25687 lb	Passed - 13%
2	4-06	1.25D + 1.5L	1.00	2929 lb		20065 lb	20433 lb	Passed - 15%

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	16'- 2 7/8"	Self Weight	Top	13 lb/ft	-	-	-
Uniform	5'- 4 7/16"	10'- 4 7/16"	Smoothed Load	Front	64 lb/ft	128 lb/ft	-	-
Point	1'- 5 1/4"	1'- 5 1/4"	J4(i50290)	Front	67 lb	178 lb	-	-
Point	2'- 9 1/4"	2'- 9 1/4"	J4(i50382)	Front	64 lb	170 lb	-	-
Point	4'- 1"	4'- 1"	B18 (-3R)(i49844)	Front	607 lb	724 lb	-	-
Point	4'- 10 7/16"	4'- 10 7/16"	J4(i50516)	Front	57 lb	115 lb	-	-
Point	10'- 10 3/8"	10'- 10 3/8"	J4(i50519)	Front	46 lb	99 lb	-	-
Point	11'- 5 1/8"	11'- 5 1/8"	J3(i49975)	Front	63 lb	126 lb	-	-
Point	12'- 3 5/8"	12'- 3 5/8"	J3(i50443)	Front	82 lb	164 lb	-	-
Point	13'- 3 5/8"	13'- 3 5/8"	J3(i49874)	Front	88 lb	175 lb	-	-
Point	14'- 3 5/8"	14'- 3 5/8"	J3(i50377)	Front	87 lb	174 lb	-	-
Point	15'- 3 5/8"	15'- 3 5/8"	J3(i50332)	Front	80 lb	160 lb	-	-

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 5 1/2"	Pt2(i50363)	971 lb	1438 lb	-	-
2	15'- 10 1/2"	16'- 2 7/8"	Pt2(i49905)	799 lb	1287 lb	-	-

DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Lateral stability factor (KL) was based on user preference to use the width of one ply.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.

PLY TO PLY CONNECTION

- Member design assumed proper ply to ply connection by others. Fastener spacing along length of member must not exceed 4 times depth of member. Verify connection between plies according to code specification and follow the manufacturer's installation instruction. Loads assumed to be distributed equally to each ply.

SEQ46627

MiTek	Customer: **Gold Park Homes** Job Address: **Pine Valley Ph2** City: **Vaughan** Job Track: **45147**	Job Name: **343077 Ground A + Second A (1,** Level: **Ground Floor** Label: **B21 (-3R) - i50402** Type: **Beam**	**1 Ply Member** **11 7/8" NI-20**	Status: **Design Passed**

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in MiTek® Structure Version 8.5.3.233.Update5.15

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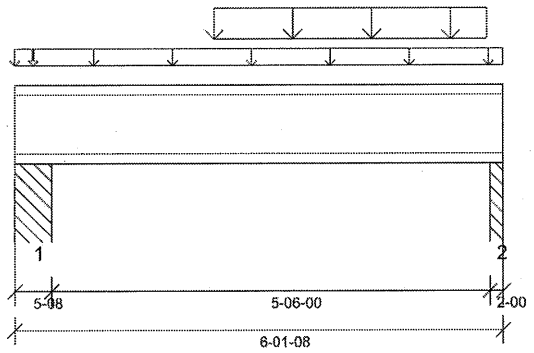


DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)
Design Methodology: LSD
Service Condition: Dry
LL Deflection Limit: L/360,
TL Deflection Limit: L/240,

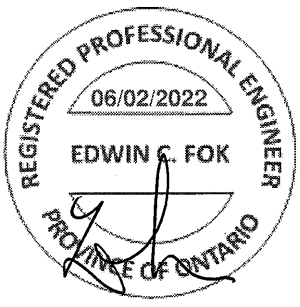
Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 5'- 10"

Factored Resistance of Support Material:

- 1334 psi Column @ 0'- 4 1/2"
- 1334 psi Column @ 6'- 1/2"



ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	3'- 6 3/4"	1.25D + 1.5L	1.00	1631 lb ft	5580 lb ft	Passed - 29%
Factored Shear:	5'- 11 7/16"	1.25D + 1.5L	1.00	1256 lb	2240 lb	Passed - 56%
Live Load (LL) Pos. Defl.:	3'- 3 7/8"	L		0.031"	L/360	Passed - L/999
Total Load (TL) Pos. Defl.:	3'- 3 7/8"	D + L		0.043"	L/240	Passed - L/999

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	5-08	1.25D + 1.5L	1.00	751 lb		2240 lb	18349 lb	Passed - 34%
2	2-00	1.25D + 1.5L	1.00	1283 lb		2000 lb	6672 lb	Passed - 64%

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	6'- 1 1/2"	Self Weight	Top	3 lb/ft	-	-	-
Uniform	0'	6'- 1 1/2"	FC1 Floor Decking (Plan View Fill)	Top	11 lb/ft	29 lb/ft	-	-
Uniform	2'- 6"	5'- 11"	User Load	Top	90 lb/ft	240 lb/ft	-	-
Point	0'- 2 3/4"	0'- 2 3/4"	Pt3(i49948)	Top	13 lb	12 lb	-	-

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 5 1/2"	Pt2(i50280)	153 lb	360 lb	-	-
2	5'- 11 1/2"	6'- 1 1/2"	Pt2(i50363)	254 lb	656 lb	-	-

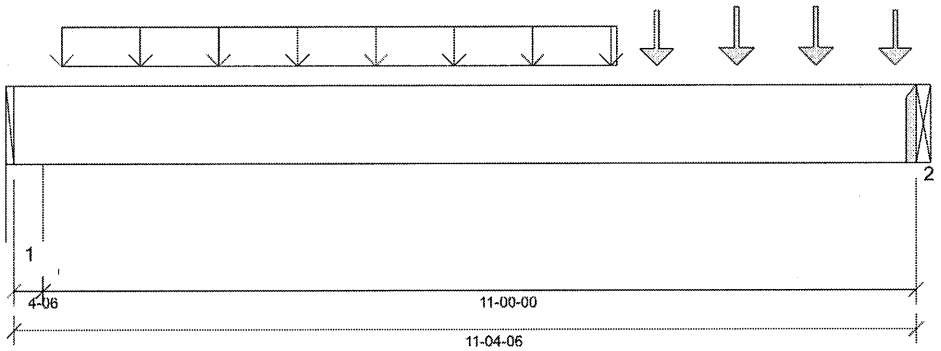
DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.

S2024668

MiTek	Customer: **Gold Park Homes** Job Address: **Pine Valley Ph2** City: **Vaughan** Job Track: **45147**	Job Name: **343077 Second A WO Loft (14)** Level: **Second Floor** Label: **B22 - i54142** Type: **Beam**	**2 Ply Member** **1 3/4" x 11 7/8" 1.55E TimberStrand® LSL**	Status: **Design Passed**

Illustration Not to Scale. Pitch: 0/12 — Designed by Single Member Design Engine in MiTek® Structure Version 8.5.3.233.Update5.15 — Report Version: 2021.03.26 05/03/2022 14:34



DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)
Design Methodology: LSD
Service Condition: Dry
LL Deflection Limit: L/360,
TL Deflection Limit: L/240,

Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 0'- 9 1/2"

Factored Resistance of Support Material:

- 615 psi Wall @ 0'- 3 3/8"
- 769 psi Beam @ 11'- 4 3/8"

NAIL ONE PLY TO ANOTHER WITH 3-1/2" SPIRAL NAILS @ 9" O/C STAGGERED IN 2 ROWS

REGISTERED PROFESSIONAL ENGINEER
06/02/2022
EDWIN C. FOK
PROVINCE OF ONTARIO

ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	6'- 1 1/4"	1.25D + 1.5L	1.00	13470 lb ft	26531 lb ft	Passed - 51%
Factored Shear:	10'- 4 1/2"	1.25D + 1.5L	1.00	4454 lb	14414 lb	Passed - 31%
Live Load (LL) Pos. Defl.:	5'- 10 3/8"	L		0.201"	L/360	Passed - L/658
Total Load (TL) Pos. Defl.:	5'- 10 3/8"	D + L		0.284"	L/240	Passed - L/464

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	4-06	1.25D + 1.5L	1.00	4443 lb		20065 lb	9420 lb	Passed - 47%
2	1-08	1.25D + 1.5L	1.00	5413 lb		6880 lb	-	Passed - 79%

CONNECTOR INFORMATION

ID	Part No.	Manufacturer	Nailing Requirements			Other Information or Requirement for Reinforcement Accessories
			Top	Face	Member	
2	HGUS410		-	-	-	Connector manually specified by the user.

* Connectors: Refer to manufacturer's specifications, fasteners requirements and installation instruction. Where header fasteners are longer than the width of the supporting member, install backer block or clinch header nails.

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	11'- 4 3/8"	Self Weight	Top	13 lb/ft	-	-	-
Uniform	0'- 7 1/4"	7'- 7 1/4"	Smoothed Load	Front	151 lb/ft	403 lb/ft	-	-
Point	8'- 1 1/4"	8'- 1 1/4"	J6(i54036)	Front	190 lb	476 lb	-	-
Point	9'- 1 1/4"	9'- 1 1/4"	J6(i54037)	Front	217 lb	508 lb	-	-
Point	10'- 1 1/4"	10'- 1 1/4"	J6(i54084)	Front	217 lb	508 lb	-	-
Point	11'- 1 1/4"	11'- 1 1/4"	J6(i54039)	Front	201 lb	461 lb	-	-

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 4 3/8"	12(i51771)	909 lb	2189 lb	-	-
2	11'- 4 3/8"	11'- 4 3/8"	B23(i53621)	1152 lb	2663 lb	-	-

DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Lateral stability factor (KL) was based on user preference to use the width of one ply.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
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- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.

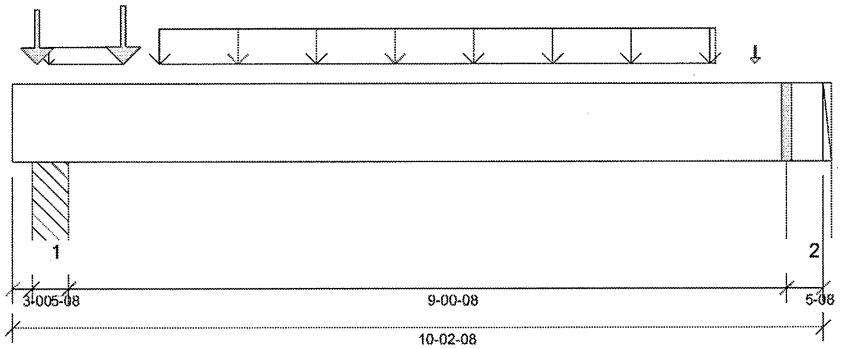
PLY TO PLY CONNECTION

- Member design assumed proper ply to ply connection by others. Fastener spacing along length of member must not exceed 4 times depth of member. Verify connection between plies according to code specification and follow the manufacturer's installation instruction. Loads assumed to be distributed equally to each ply.

S6040679

MiTek	Customer: **Gold Park Homes** Job Address: **Pine Valley Ph2** City: **Vaughan** Job Track: **45147**	Job Name: **343077 Second A WO Loft (14)** Level: **Second Floor** Label: **B23 - i53621** Type: **Beam**	**2 Ply Member** **1 3/4" x 11 7/8" 1.55E TimberStrand® LSL**	Status: **Design Passed**

Illustration Not to Scale. Pitch: 0/12 — Designed by Single Member Design Engine in MiTek® Structure Version 8.5.3.233.Update5.15 — Report Version: 2021.03.26 05/03/2022 14:35



DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)
Design Methodology: LSD
Service Condition: Dry
LL Deflection Limit: L/360,
TL Deflection Limit: L/240,

Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 0'- 9 1/2"

Factored Resistance of Support Material:

- 1334 psi Column @ 0'- 5 3/4"
- 615 psi Wall @ 9'- 10"

NAIL ONE PLY TO ANOTHER WITH 3-1/2" SPIRAL NAILS @12" O/C STAGGERED IN 2 ROWS

ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	4'- 4 1/4"	1.25D + 1.5L	1.00	8884 lb ft	26531 lb ft	Passed - 33%
Factored Neg. Moment:	0'- 5 3/4"	1.25D + 1.5L	1.00	923 lb ft	26531 lb ft	Passed - 3%
Factored Shear:	1'- 8 3/8"	1.25D + 1.5L	1.00	7523 lb	14414 lb	Passed - 52%
Live Load (LL) Pos. Defl.:	4'- 11 5/8"	L		0.086"	L/360	Passed - L/999
Total Load (TL) Pos. Defl.:	4'- 11 13/16"	D + L		0.138"	L/240	Passed - L/786

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	5-08	1.25D + 1.5L	1.00	13135 lb		25225 lb	25687 lb	Passed - 52%
2	5-08	1.25D + 1.5L	1.00	3183 lb		25226 lb	11843 lb	Passed - 27%

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	10'- 2 1/2"	Self Weight	Top	13 lb/ft	-	-	-
Uniform	0'- 5 1/2"	1'- 4 3/4"	FC3 Floor Decking (Plan View Fill)	Top	51 lb/ft	-	-	-
Uniform	1'- 10 1/4"	8'- 10 1/4"	Smoothed Load	Front	159 lb/ft	264 lb/ft	-	-
Point	1'- 4 3/4"	1'- 4 3/4"	B6(i54076)	Front	1344 lb	2840 lb	-	-
Point	9'- 4 1/4"	9'- 4 1/4"	J2(i54010)	Front	104 lb	197 lb	-	-
Point	0'- 3 3/4"	0'- 3 3/4"	B22(i54142)	Back	1152 lb	2663 lb	-	-

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'- 3"	0'- 8 1/2"	Pt1(i50787)	3054 lb	6148 lb	-	-
2	9'- 9"	10'- 2 1/2"	9(i41702)	903 lb	1433 lb	-	-

DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Lateral stability factor (KL) was based on user preference to use the width of one ply.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- The deflection at the cantilever for either live and/or total loads is less than 3/8" and therefore has been excluded from the deflection ratio considerations.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.
- Bearing capacity of member at support 1, 2 was verified for the effect of concentrated load applied near the support. At support 1. Required Load Area: L=3.500", W=3.500". LDF=1.00, Pf=5940 lb, Q'r=13759 lb, Result=43.17%.

PLY TO PLY CONNECTION

- Member design assumed proper ply to ply connection by others. Fastener spacing along length of member must not exceed 4 times depth of member. Verify connection between plies according to code specification and follow the manufacturer's installation instruction. Loads assumed to be distributed equally to each ply.

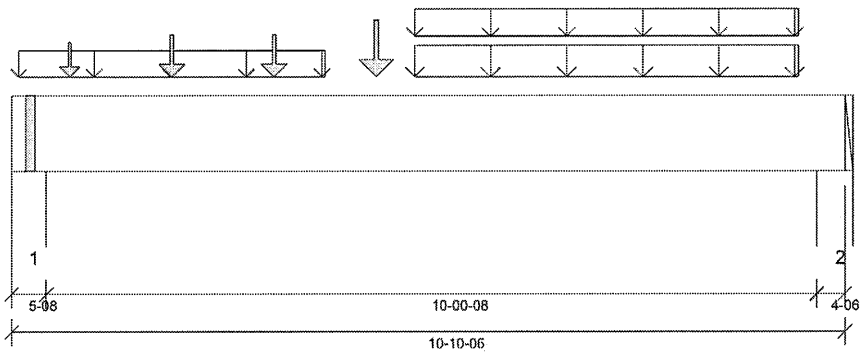
SG04660

MiTek	Customer: **Gold Park Homes** Job Address: **Pine Valley Ph2** City: **Vaughan** Job Track: **45147**	Job Name: **343077 Ground A + Second A...** Level: **Second Floor** Label: **B24 - i54731** Type: **Beam**	**2 Ply Member** **1 3/4" x 11 7/8" 1.55E TimberStrand® LSL**	Status: **Design Passed**

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in MiTek® Structure Version 8.5.3.233.Update5.15

Report Version: 2021.03.26 05/03/2022 19:44



DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)
Design Methodology: LSD
Service Condition: Dry
LL Deflection Limit: L/360,
TL Deflection Limit: L/240,

Lateral Restraint Requirements:

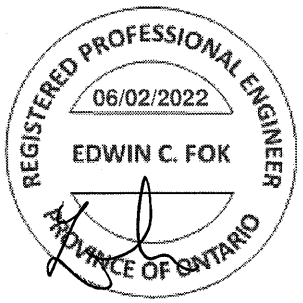
Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 1'- 1 1/2"

Factored Resistance of Support Material:

- 615 psi Wall @ 0'- 4 1/2"
- 615 psi Wall @ 10'- 7"

NAIL ONE PLY TO ANOTHER WITH 3-1/2" SPIRAL NAILS @ 12" O/C STAGGERED IN 2 ROWS



ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	5'- 9 1/8"	1.25D + 1.5L	1.00	11938 lb ft	26531 lb ft	Passed - 45%
Factored Shear:	1'- 5 3/8"	1.25D + 1.5L	1.00	4649 lb	14414 lb	Passed - 32%
Live Load (LL) Pos. Defl.:	5'- 5 13/16"	L		0.148"	L/360	Passed - L/816
Total Load (TL) Pos. Defl.:	5'- 5 15/16"	D + L		0.213"	L/240	Passed - L/566

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	5-08	1.25D + 1.5L	1.00	4673 lb		25225 lb	11842 lb	Passed - 39%
2	4-06	1.25D + 1.5L	1.00	4410 lb		20065 lb	9420 lb	Passed - 47%

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	10'- 10 3/8"	Self Weight	Top	13 lb/ft	-	-	-
Uniform	0'- 1 1/8"	4'- 1 1/8"	Smoothed Load	Front	70 lb/ft	187 lb/ft	-	-
Uniform	5'- 3 1/8"	10'- 3 1/8"	Smoothed Load	Back	104 lb/ft	258 lb/ft	-	-
Uniform	5'- 3 1/8"	10'- 3 1/8"	Smoothed Load	Front	91 lb/ft	187 lb/ft	-	-
Point	4'- 9 1/8"	4'- 9 1/8"	-	Front	200 lb	519 lb	-	-
Point	0'- 9 1/8"	0'- 9 1/8"	J2(i54765)	Back	94 lb	252 lb	-	-
Point	2'- 1 1/8"	2'- 1 1/8"	J2(i54839)	Back	129 lb	344 lb	-	-
Point	3'- 5 1/8"	3'- 5 1/8"	J2(i54852)	Back	129 lb	344 lb	-	-

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 5 1/2"	24(i51833)	975 lb	2302 lb	-	-
2	10'- 6"	10'- 10 3/8"	E25(i41701)	974 lb	2129 lb	-	-

DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Lateral stability factor (KL) was based on user preference to use the width of one ply.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.

PLY TO PLY CONNECTION

- Member design assumed proper ply to ply connection by others. Fastener spacing along length of member must not exceed 4 times depth of member. Verify connection between plies according to code specification and follow the manufacturer's installation instruction. Loads assumed to be distributed equally to each ply.

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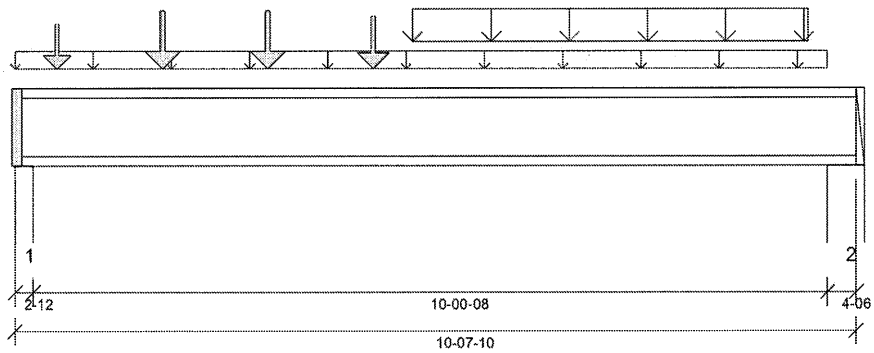
MiTek	Customer: **Gold Park Homes** Job Address: **Pine Valley Ph2** City: **Vaughan** Job Track: **45147**	Job Name: **343077 Ground A + Second A...** Level: **Second Floor** Label: **B25 - i54388** Type: **Beam**	**2 Ply Member** **11 7/8" NI-20**	Status: **Design Passed**

Illustration Not to Scale. Pitch: 0/12 | Designed by Single Member Design Engine in MiTek® Structure Version 8.5.3.233.Update5.15 | Report Version: 2021.03.26 05/03/2022 19:44



DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)
Design Methodology: LSD
Service Condition: Dry
LL Deflection Limit: L/360,
TL Deflection Limit: L/240,

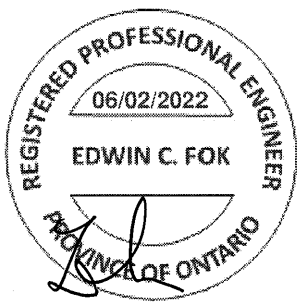
Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 1'- 1 1/2"

Factored Resistance of Support Material:

- 615 psi Wall @ 0'- 1 3/4"
- 615 psi Wall @ 10'- 4 1/4"



ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	5'- 6 3/8"	1.25D + 1.5L	1.00	7975 lb ft	11160 lb ft	Passed - 71%
Factored Shear:	0'- 2 13/16"	1.25D + 1.5L	1.00	3054 lb	4480 lb	Passed - 68%
Live Load (LL) Pos. Defl.:	5'- 3 1/16"	L		0.156"	L/360	Passed - L/769
Total Load (TL) Pos. Defl.:	5'- 3 1/16"	D + L		0.254"	L/240	Passed - L/475

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	2-12	1.25D + 1.5L	1.00	3073 lb		4180 lb	8459 lb	Passed - 74%
2	4-06	1.25D + 1.5L	1.00	2946 lb		4480 lb	13457 lb	Passed - 66%

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	10'- 7 5/8"	Self Weight	Top	6 lb/ft	-	-	-
Uniform	0'	10'- 3 1/4"	User Load	Top	60 lb/ft	-	-	-
Uniform	5'- 3/8"	10'- 3/8"	Smoothed Load	Front	101 lb/ft	269 lb/ft	-	-
Point	0'- 6 3/8"	0'- 6 3/8"	J2(i54765)	Front	94 lb	249 lb	-	-
Point	1'- 10 3/8"	1'- 10 3/8"	J2(i54839)	Front	135 lb	359 lb	-	-
Point	3'- 2 3/8"	3'- 2 3/8"	J2(i54852)	Front	135 lb	359 lb	-	-
Point	4'- 6 3/8"	4'- 6 3/8"	J2(i54657)	Front	118 lb	314 lb	-	-

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 2 3/4"	9(i41702)	849 lb	1341 lb	-	-
2	10'- 3 1/4"	10'- 7 5/8"	E25(i41701)	815 lb	1285 lb	-	-

DESIGN NOTES

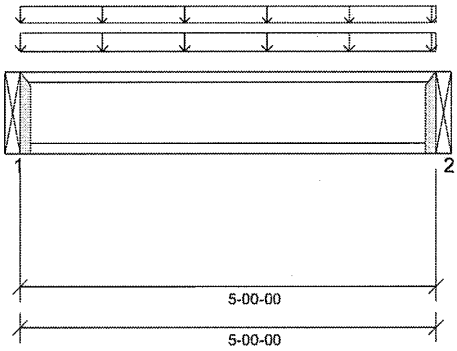
- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.

PLY TO PLY CONNECTION

- Member design assumed proper ply to ply connection by others. Verify connection between plies according to code specification and follow the manufacturer's installation instruction. Loads assumed to be distributed equally to each ply.

MiTek	Customer: **Gold Park Homes** Job Address: **Pine Valley Ph2** City: **Vaughan** Job Track: **45147**	Job Name: **343077 Ground A + Second A...** Level: **Second Floor** Label: **B26 - i54803** Type: **Beam**	**1 Ply Member** **11 7/8" NI-20**	Status: **Design Passed**

Illustration Not to Scale. Pitch: 0/12 | Designed by Single Member Design Engine in MiTek® Structure Version 8.5.3.233.Update5.15 | Report Version: 2021.03.26 05/03/2022 19:44



DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)
Design Methodology: LSD
Service Condition: Dry
LL Deflection Limit: L/360,
TL Deflection Limit: L/240,

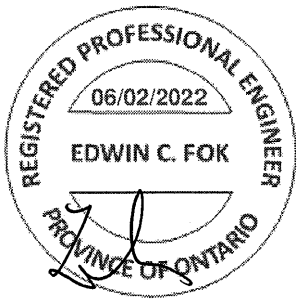
Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 5'

Factored Resistance of Support Material:

- 769 psi Beam @ 0'
- 769 psi Beam @ 5'



ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	2'- 6"	1.25D + 1.5L	0.72	369 lb ft	4021 lb ft	Passed - 9%
Factored Shear:	0'- 1/16"	1.25D + 1.5L	0.72	294 lb	1614 lb	Passed - 18%

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	1-12	1.25D + 1.5L	0.72	295 lb		1970 lb	-	Passed - 15%
2	1-12	1.25D + 1.5L	0.72	295 lb		1970 lb	-	Passed - 15%

CONNECTOR INFORMATION

ID	Part No.	Manufacturer	Nailing Requirements			Other Information or Requirement for Reinforcement Accessories
			Top	Face	Member	
1	LT251188		-	-	-	Connector manually specified by the user.
2	LT251188		-	-	-	Connector manually specified by the user.

* Connectors: Refer to manufacturer's specifications, fasteners requirements and installation instruction. Where header fasteners are longer than the width of the supporting member, install backer block or clinch header nails.

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	5'	Self Weight	Top	3 lb/ft	-	-	-
Uniform	-0'	5'	19(i51781)	Top	61 lb/ft	-	-	-
Uniform	0'	5'	FC3 Floor Decking (Plan View Fill)	Top	7 lb/ft	20 lb/ft	-	-

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'	B29(i54286)	177 lb	49 lb	-	-
2	5'	5'	B27(i54263)	177 lb	49 lb	-	-

DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.

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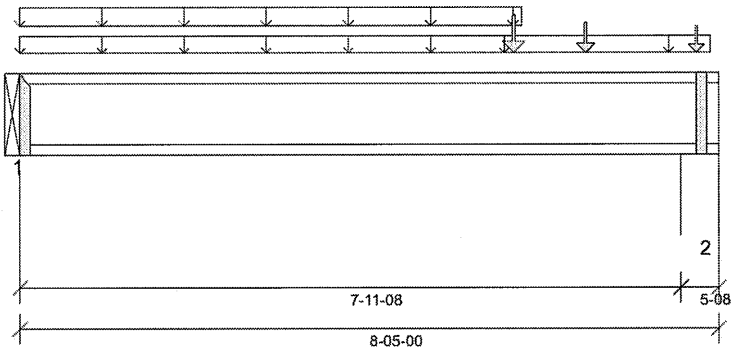
MiTek	Customer: **Gold Park Homes** Job Address: **Pine Valley Ph2** City: **Vaughan** Job Track: **45147**	Job Name: **343077 Ground A + Second A...** Level: **Second Floor** Label: **B27 - i54263** Type: **Beam**	**1 Ply Member** **11 7/8" NI-20**	Status: **Design Passed**

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in MiTek® Structure Version 8.5.3.233.Update5.15

Report Version: 2021.03.26 05/03/2022 19:45



DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)
Design Methodology: LSD
Service Condition: Dry
LL Deflection Limit: L/360,
TL Deflection Limit: L/240,

Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 5'- 10"

Factored Resistance of Support Material:

- 769 psi Beam @ 0'
- 615 psi Wall @ 8'- 1/2"

ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	4'- 7 1/16"	1.4D	0.65	1062 lb ft	3627 lb ft	Passed - 29%
Factored Shear:	7'- 11 7/16"	1.25D + 1.5L	0.89	786 lb	1991 lb	Passed - 40%
Live Load (LL) Pos. Defl.:	4'- 2 7/8"	L		0.019"	L/360	Passed - L/999
Total Load (TL) Pos. Defl.:	4'- 1 7/8"	D + L		0.066"	L/240	Passed - L/999

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	1-12	1.4D	0.65	463 lb		1970 lb	-	Passed - 23%
2	5-08	1.25D + 1.5L	0.89	974 lb		1991 lb	7519 lb	Passed - 49%

CONNECTOR INFORMATION

ID	Part No.	Manufacturer	Nailing Requirements			Other Information or Requirement for Reinforcement Accessories
			Top	Face	Member	
1	LT251188		-	-	-	Connector manually specified by the user.

* Connectors: Refer to manufacturer's specifications, fasteners requirements and installation instruction. Where header fasteners are longer than the width of the supporting member, install backer block or clinch header nails.

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	8'- 5"	Self Weight	Top	3 lb/ft	-	-	-
Uniform	0'	6'- 1/2"	22(i51778)	Top	61 lb/ft	-	-	-
Uniform	0'	5'- 10"	FC3 Floor Decking (Plan View Fill)	Top	8 lb/ft	22 lb/ft	-	-
Uniform	5'- 10"	8'- 3 3/4"	FC3 Floor Decking (Plan View Fill)	Top	8 lb/ft	20 lb/ft	-	-
Point	5'- 11 5/16"	5'- 11 5/16"	-	Back	190 lb	49 lb	-	-
Point	6'- 9 3/4"	6'- 9 3/4"	J7(i54649)	Back	44 lb	118 lb	-	-
Point	8'- 1 3/4"	8'- 1 3/4"	J7(i54650)	Back	33 lb	87 lb	-	-

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'	B30(i54763)	338 lb	120 lb	-	-
2	7'- 11 1/2"	8'- 5"	24(i51833)	390 lb	316 lb	-	-

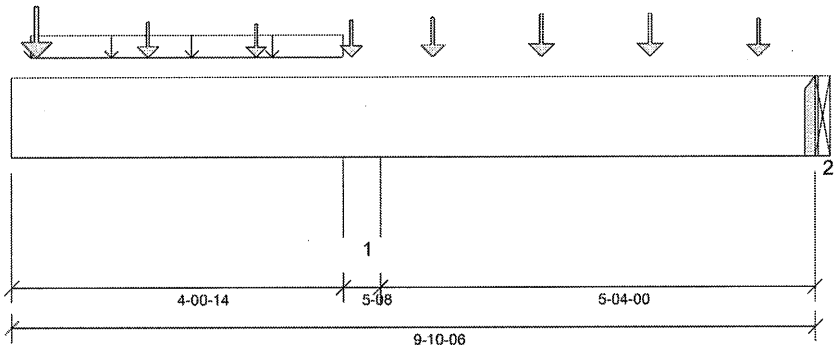
DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.

SZ04668Q

MiTek	Customer: **Gold Park Homes** Job Address: **Pine Valley Ph2** City: **Vaughan** Job Track: **45147**	Job Name: **343077 Ground A + Second A...** Level: **Second Floor** Label: **B28 (CANT.) - i54890** Type: **Beam**	**2 Ply Member** **1 3/4" x 11 7/8" 1.55E TimberStrand® LSL**	Status: **Design Passed**

Illustration Not to Scale. Pitch: 0/12 — Designed by Single Member Design Engine in MiTek® Structure Version 8.5.3.233.Update5.15 — Report Version: 2021.03.26 05/03/2022 19:45



DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)
Design Methodology: LSD
Service Condition: Dry
LL Deflection Limit: L/360,
TL Deflection Limit: L/240,

Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0'- 2 7/8" Bottom: 1'- 2 1/4"

Factored Resistance of Support Material:

- 615 psi Wall @ 4'- 3 5/8"
- 769 psi Beam @ 9'- 10 3/8"

NAIL ONE PLY TO ANOTHER WITH 3-1/2" SPIRAL NAILS @ 12" O/C STAGGERED IN 2 ROWS

REGISTERED PROFESSIONAL ENGINEER
06/02/2022
EDWIN C. FOK
PROVINCE OF ONTARIO

ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	7'- 10 1/8"	0.9D + 1.5L	0.96	760 lb ft	25348 lb ft	Passed - 3%
Factored Neg. Moment:	4'- 3 5/8"	1.25D + 1.5L	1.00	5317 lb ft	25719 lb ft	Passed - 21%
Factored Shear:	3'- 1"	1.25D + 1.5L	1.00	1795 lb	14414 lb	Passed - 12%
Live Load (LL) Neg. Defl.:	6'- 7 13/16"	L		0.012"	L/360	Passed - L/999
Total Load (TL) Neg. Defl.:	6'- 7 1/16"	D + L		0.015"	L/240	Passed - L/999

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	5-08	1.25D + 1.5L	1.00	4143 lb		25225 lb	11843 lb	Passed - 35%
2	1-08	1.25D + 1.5L	0.96	603 lb		603 lb	-	Passed - 100%
2	1-08	0.9D + 1.5L	1.00		-705 lb	-	-	

CONNECTOR INFORMATION

ID	Part No.	Manufacturer	Nailing Requirements			Other Information or Requirement for Reinforcement Accessories
			Top	Face	Member	
2	HU410		-	-	-	Connector manually specified by the user.

* Connectors: Refer to manufacturer's specifications, fasteners requirements and installation instruction. Where header fasteners are longer than the width of the supporting member, install backer block or clinch header nails.

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	-0'	9'- 10 3/8"	Self Weight	Top	13 lb/ft	-	-	-
Uniform	0'- 2 7/8"	4'- 7/8"	User Load	Top	38 lb/ft	100 lb/ft	-	-
Point	0'- 3 3/4"	0'- 3 3/4"	B5(i54715)	Front	132 lb	270 lb	-	-
Point	1'- 8 1/8"	1'- 8 1/8"	J6(i54899)	Front	61 lb	163 lb	-	-
Point	3'- 1/8"	3'- 1/8"	J6(i54447)	Front	56 lb	148 lb	-	-
Point	5'- 2 1/8"	5'- 2 1/8"	-	Front	74 lb	197 lb	-	-
Point	6'- 6 1/8"	6'- 6 1/8"	-	Front	83 lb	222 lb	-	-
Point	7'- 10 1/8"	7'- 10 1/8"	-	Front	83 lb	222 lb	-	-
Point	9'- 2 1/8"	9'- 2 1/8"	-	Front	69 lb	184 lb	-	-
Point	4'- 2 1/8"	4'- 2 1/8"	B2(i54372)	Back	183 lb	57 lb	-	-

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	4'- 7/8"	4'- 6 3/8"	10(i41708)	1039 lb	1887 lb	-	-
2	9'- 10 3/8"	9'- 10 3/8"	B29(i54286)	-19 lb	422/-450 lb	-	-

DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Lateral stability factor (KL) was based on user preference to use the width of one ply.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- The deflection at the cantilever for either live and/or total loads is less than 3/8" and therefore has been excluded from the deflection ratio considerations.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.

MiTek	Customer: **Gold Park Homes** Job Address: **Pine Valley Ph2** City: **Vaughan** Job Track: **45147**	Job Name: **343077 Ground A + Second A...** Level: **Second Floor** Label: **B29 - i54286** Type: **Beam**	**1 Ply Member** **11 7/8" NI-20**	Status: **Design Passed**

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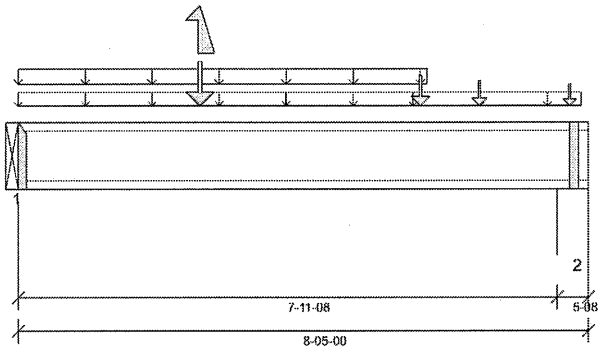


DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)
Design Methodology: LSD
Service Condition: Dry
LL Deflection Limit: L/360,
TL Deflection Limit: L/240,

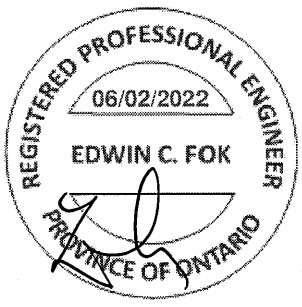
Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 3'

Factored Resistance of Support Material:

- 769 psi Beam @ 0'
- 615 psi Wall @ 8'- 1/2"



ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	3'- 1"	1.25D + 1.5L	1.00	2183 lb ft	5580 lb ft	Passed - 39%
Factored Neg. Moment:	2'- 8 1/4"	0.9D + 1.5L	0.91	684 lb ft	5053 lb ft	Passed - 14%
Factored Shear:	0'- 1/16"	1.25D + 1.5L	1.00	962 lb	2240 lb	Passed - 43%
Live Load (LL) Pos. Defl.:	3'- 10 3/16"	L		0.055"	L/360	Passed - L/999
Total Load (TL) Pos. Defl.:	3'- 11 13/16"	D + L		0.097"	L/240	Passed - L/984

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	1-12	1.25D + 1.5L	1.00	963 lb		1970 lb	-	Passed - 49%
1	1-12	0.9D + 1.5L	0.91		-170 lb	-	-	
2	5-08	1.25D + 1.5L	1.00	1142 lb		2240 lb	8459 lb	Passed - 51%

CONNECTOR INFORMATION

ID	Part No.	Manufacturer	Nailing Requirements			Other Information or Requirement for Reinforcement Accessories
			Top	Face	Member	
1	LT251188		-	-	-	Connector manually specified by the user.

* Connectors: Refer to manufacturer's specifications, fasteners requirements and installation instruction. Where header fasteners are longer than the width of the supporting member, install backer block or clinch header nails.

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	8'- 5"	Self Weight	Top	3 lb/ft	-	-	-
Uniform	0'	6'- 1/2"	20(i51780)	Top	61 lb/ft	-	-	-
Uniform	0'	5'- 10"	FC3 Floor Decking (Plan View Fill)	Top	7 lb/ft	18 lb/ft	-	-
Uniform	5'- 10"	8'- 3 3/4"	FC3 Floor Decking (Plan View Fill)	Top	6 lb/ft	16 lb/ft	-	-
Point	5'- 11 5/16"	5'- 11 5/16"	-	Front	191 lb	49 lb	-	-
Point	6'- 9 3/4"	6'- 9 3/4"	J7(i54649)	Front	44 lb	118 lb	-	-
Point	8'- 1 3/4"	8'- 1 3/4"	J7(i54650)	Front	33 lb	87 lb	-	-
Point	2'- 8 1/4"	2'- 8 1/4"	B28 (CANT.) (i54890)	Back	-19 lb	422/-450 lb	-	-

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'	B30(i54763)	317 lb	385/-300 lb	-	-
2	7'- 11 1/2"	8'- 5"	25(i51910)	379 lb	439/-150 lb	-	-

DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.

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MiTek	Customer: **Gold Park Homes** Job Address: **Pine Valley Ph2** City: **Vaughan** Job Track: **45147**	Job Name: **343077 Ground A + Second A...** Level: **Second Floor** Label: **B30 - i54763** Type: **Beam**	**3 Ply Member** **1 3/4" x 11 7/8" 1.55E TimberStrand® LSL**	Status: **Design Passed**

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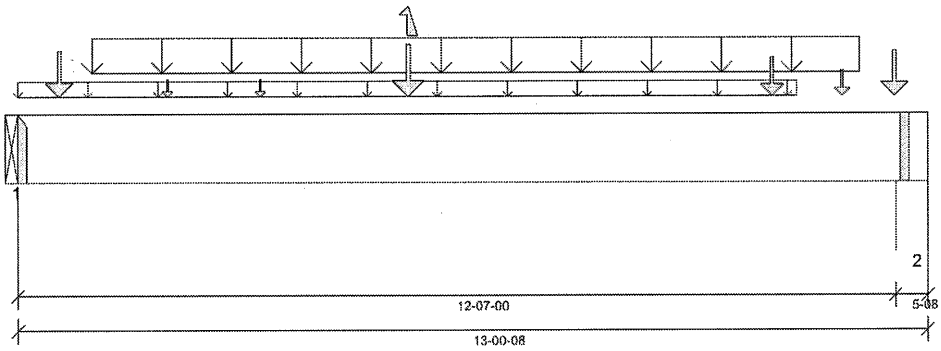

DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)
Design Methodology: LSD
Service Condition: Dry
LL Deflection Limit: L/360,
TL Deflection Limit: L/240,

Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 0'- 9 1/2"

Factored Resistance of Support Material:

- 769 psi Beam @ 0'
- 615 psi Wall @ 12'- 8"

NAIL ONE PLY TO ANOTHER WITH 3-1/2" SPIRAL NAILS @ 6 " O/C STAGGERED IN 2 ROWS

ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	5'- 7 1/4"	1.25D + 1.5L	1.00	24190 lb ft	39797 lb ft	Passed - 61%
Factored Shear:	11'- 7 1/8"	1.25D + 1.5L	1.00	7828 lb	21621 lb	Passed - 36%
Live Load (LL) Pos. Defl.:	6'- 3 5/8"	L		0.270"	L/360	Passed - L/559
Total Load (TL) Pos. Defl.:	6'- 3 13/16"	D + L		0.447"	L/240	Passed - L/337
Permanent Deflection:	6'- 4"			-	L/360	Passed - L/877

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	1-08	1.25D + 1.5L	1.00	7153 lb		10319 lb	-	Passed - 69%
2	5-08	1.25D + 1.5L	1.00	7937 lb		37838 lb	17764 lb	Passed - 45%

CONNECTOR INFORMATION

ID	Part No.	Manufacturer	Nailing Requirements			Other Information or Requirement for Reinforcement Accessories
			Top	Face	Member	
1	HGUS5.50/10		-	-	-	Connector manually specified by the user.

* Connectors: Refer to manufacturer's specifications, fasteners requirements and installation instruction. Where header fasteners are longer than the width of the supporting member, install backer block or clinch header nails.

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	13'- 1/2"	Self Weight	Top	19 lb/ft	-	-	-
Uniform	0'	11'- 2"	-	Top	60 lb/ft	-	-	-
Uniform	1'- 3/4"	12'- 3/4"	Smoothed Load	Front	187 lb/ft	432 lb/ft	-	-
Point	0'- 7 1/8"	0'- 7 1/8"	-	Front	159 lb	434 lb	-	-
Point	12'- 6 3/4"	12'- 6 3/4"	J11(i54564)	Front	164 lb	411 lb	-	-
Point	2'- 1 3/4"	2'- 1 3/4"	J9(i54678)	Back	-	79 lb	-	-
Point	3'- 5 3/4"	3'- 5 3/4"	J9(i54856)	Back	-	79 lb	-	-
Point	5'- 7 1/4"	5'- 7 1/4"	B29(i54286)	Back	317 lb	385/-300 lb	-	-
Point	10'- 9 3/4"	10'- 9 3/4"	B27(i54263)	Back	338 lb	120 lb	-	-
Point	11'- 9 3/4"	11'- 9 3/4"	J5(i54672)	Back	72 lb	156 lb	-	-

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'	B31(i54841)	1967 lb	3077/-165 lb	-	-
2	12'- 7"	13'- 1/2"	-	2258 lb	3462/-135 lb	-	-
++>	12'- 10 1/4"	12'- 10 1/4"	4(i41664)	1828 lb	2803/-109 lb	-	-
++>	13'	13'	3(i41663)	430 lb	659/-26 lb	-	-

DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Lateral stability factor (KL) was based on user preference to use the width of one ply.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.

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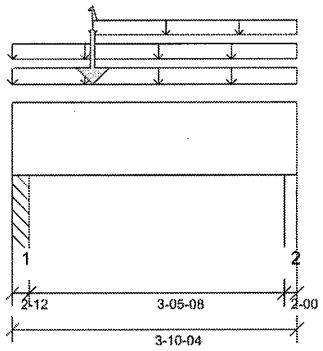
MiTek	Customer: **Gold Park Homes** Job Address: **Pine Valley Ph2** City: **Vaughan** Job Track: **45147**	Job Name: **343077 Ground A + Second A...** Level: **Second Floor** Label: **B31 - i54841** Type: **Beam**	**2 Ply Member** **1 3/4" x 11 7/8" 1.55E TimberStrand® LSL**	Status: **Design Passed**

Illustration Not to Scale. Pitch: 0/12 | Designed by Single Member Design Engine in MiTek® Structure Version 8.5.3.233.Update5.15 | Report Version: 2021.03.26 05/03/2022 19:46



DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)
Design Methodology: LSD
Service Condition: Dry
LL Deflection Limit: L/360,
TL Deflection Limit: L/240,

Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 2'- 6 1/2"

Factored Resistance of Support Material:

- 1334 psi Column @ 0'- 1 3/4"
- 615 psi Wall @ 3'- 9 1/4"

NAIL ONE PLY TO ANOTHER WITH 3-1/2" SPIRAL NAILS @ 6" O/C STAGGERED IN 2 ROWS

REGISTERED PROFESSIONAL ENGINEER
06/02/2022
EDWIN C. FOK
PROVINCE OF ONTARIO

ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	1'- 1 1/8"	1.25D + 1.5L	1.00	5171 lb ft	26531 lb ft	Passed - 19%
Factored Shear:	1'- 2 5/8"	1.25D + 1.5L	1.00	4462 lb	14414 lb	Passed - 31%

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	2-12	1.25D + 1.5L	1.00	5561 lb		12613 lb	12844 lb	Passed - 44%
2	2-00	1.25D + 1.5L	1.00	2196 lb		9171 lb	4306 lb	Passed - 51%

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	3'- 10 1/4"	Self Weight	Top	13 lb/ft	-	-	-
Uniform	0'	3'- 10 1/4"	User Load	Top	60 lb/ft	-	-	-
Uniform	0'	3'- 10 1/4"	FC3 Floor Decking (Plan View Fill)	Top	9 lb/ft	24 lb/ft	-	-
Uniform	1'- 1 1/8"	3'- 10 1/4"	FC3 Floor Decking (Plan View Fill)	Top	-	19 lb/ft	-	-
Point	1'- 1 1/8"	1'- 1 1/8"	B30(i54763)	Front	1967 lb	3077/-165 lb	-	-

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 2 3/4"	Pt1(i54208)	1620 lb	2343/-121 lb	-	-
2	3'- 8 1/4"	3'- 10 1/4"	10(i41708)	692 lb	902/-44 lb	-	-

DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Lateral stability factor (KL) was based on user preference to use the width of one ply.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.
- User loads assume a bearing length of 3.5" in determining member capacity for loads near supports.
- Bearing capacity of member at support 1, 2 was verified for the effect of concentrated load applied near the support. At support 1. Required Load Area: L=3.500", W=3.500". LDF=1.00, Pf=6827 lb, Q'r=9555 lb, Result=71.45%.

PLY TO PLY CONNECTION

- Member design assumed proper ply to ply connection by others. Fastener spacing along length of member must not exceed 4 times depth of member. Verify connection between plies according to code specification and follow the manufacturer's installation instruction. Loads assumed to be distributed equally to each ply.

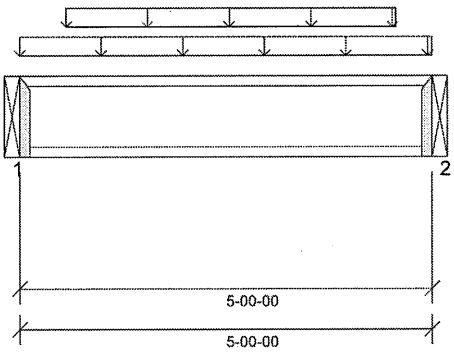
SE-046688

MiTek	Customer: **Gold Park Homes** Job Address: **Pine Valley Ph2** City: **Vaughan** Job Track: **45147**	Job Name: **343077 Ground A + Second A...** Level: **Ground Floor** Label: **B32 - i54376** Type: **Beam**	**1 Ply Member** **11 7/8" NI-20**	Status: **Design Passed**

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in MiTek® Structure Version 8.5.3.233.Update5.15

Report Version: 2021.03.26 05/03/2022 19:46



DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)
Design Methodology: LSD
Service Condition: Dry
LL Deflection Limit: L/360,
TL Deflection Limit: L/240,

Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 1'- 1 1/2"

Factored Resistance of Support Material:

- 769 psi Beam @ 0'
- 769 psi Beam @ 5'

ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	2'- 6 3/4"	1.25D + 1.5L	0.84	605 lb ft	4679 lb ft	Passed - 13%
Factored Shear:	4'- 11 15/16"	1.25D + 1.5L	0.84	427 lb	1878 lb	Passed - 23%
Total Load (TL) Pos. Defl.:	2'- 6"	D + L		0.015"	L/240	Passed - L/999

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	1-12	1.25D + 1.5L	0.84	423 lb		1970 lb	-	Passed - 21%
2	1-12	1.25D + 1.5L	0.84	428 lb		1970 lb	-	Passed - 22%

CONNECTOR INFORMATION

ID	Part No.	Manufacturer	Nailing Requirements			Other Information or Requirement for Reinforcement Accessories
			Top	Face	Member	
1	LT251188		-	-	-	Connector manually specified by the user.
2	LT251188		-	-	-	Connector manually specified by the user.

* Connectors: Refer to manufacturer's specifications, fasteners requirements and installation instruction. Where header fasteners are longer than the width of the supporting member, install backer block or clinch header nails.

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	5'	Self Weight	Top	3 lb/ft	-	-	-
Uniform	-0'	5'	15(i51774)	Top	68 lb/ft	-	-	-
Tapered	0'- 6 3/4"	4'- 6 3/4"	Smoothed Load	Back	21 To 18 lb/ft	54 To 49 lb/ft	-	-

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'	B36(i54820)	216 lb	102 lb	-	-
2	5'	5'	B34(i54292)	217 lb	104 lb	-	-

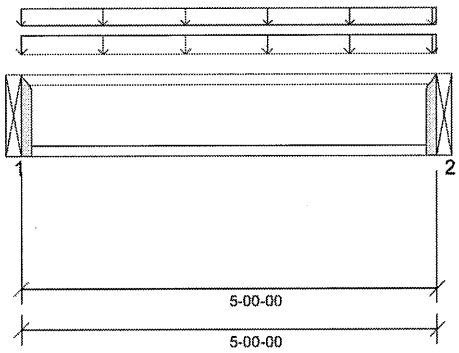
DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.

SC046689

MiTek	Customer: **Gold Park Homes** Job Address: **Pine Valley Ph2** City: **Vaughan** Job Track: **45147**	Job Name: **343077 Ground A + Second A...** Level: **Ground Floor** Label: **B33 - i54203** Type: **Beam**	**1 Ply Member** **11 7/8" NI-20**	Status: **Design Passed**

Illustration Not to Scale. Pitch: 0/12 | Designed by Single Member Design Engine in MiTek® Structure Version 8.5.3.233.Update5.15 | Report Version: 2021.03.26 05/03/2022 19:46



DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)
Design Methodology: LSD
Service Condition: Dry
LL Deflection Limit: L/360,
TL Deflection Limit: L/240,

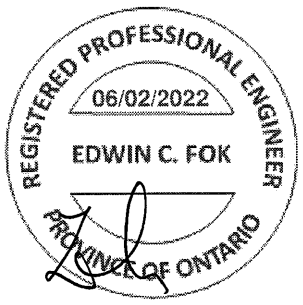
Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 5'

Factored Resistance of Support Material:

- 769 psi Beam @ 0'
- 769 psi Beam @ 5'



ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	2'- 6"	1.25D + 1.5L	0.74	424 lb ft	4109 lb ft	Passed - 10%
Factored Shear:	4'- 11 15/16"	1.25D + 1.5L	0.74	338 lb	1650 lb	Passed - 21%
Total Load (TL) Pos. Defl.:	2'- 6"	D + L		0.011"	L/240	Passed - L/999

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	1-12	1.25D + 1.5L	0.74	339 lb		1970 lb	-	Passed - 17%
2	1-12	1.25D + 1.5L	0.74	339 lb		1970 lb	-	Passed - 17%

CONNECTOR INFORMATION

ID	Part No.	Manufacturer	Nailing Requirements			Other Information or Requirement for Reinforcement Accessories
			Top	Face	Member	
1	LT251188		-	-	-	Connector manually specified by the user.
2	LT251188		-	-	-	Connector manually specified by the user.

* Connectors: Refer to manufacturer's specifications, fasteners requirements and installation instruction. Where header fasteners are longer than the width of the supporting member, install backer block or clinch header nails.

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	5'	Self Weight	Top	3 lb/ft	-	-	-
Uniform	-0'	5'	17(i51776)	Top	68 lb/ft	-	-	-
Uniform	0'	5'	FC2 Floor Decking (Plan View Fill)	Top	9 lb/ft	24 lb/ft	-	-

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'	B36(i54820)	200 lb	60 lb	-	-
2	5'	5'	B34(i54292)	200 lb	59 lb	-	-

DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.

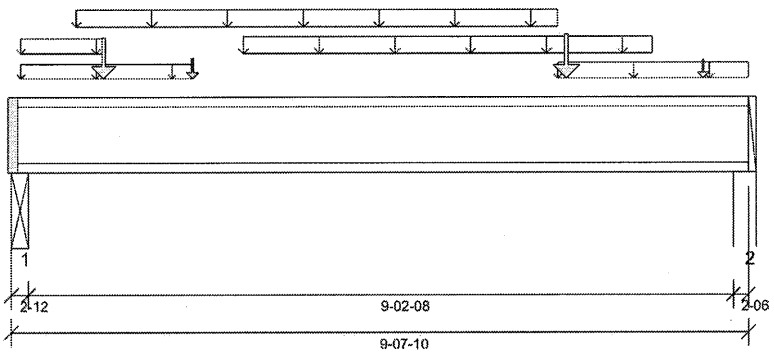
SE046690

MiTek	Customer: **Gold Park Homes** Job Address: **Pine Valley Ph2** City: **Vaughan** Job Track: **45147**	Job Name: **343077 Ground A + Second A...** Level: **Ground Floor** Label: **B34 - i54292** Type: **Beam**	**1 Ply Member** **11 7/8" NI-20**	Status: **Design Passed**

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in MiTek® Structure Version 8.5.3.233.Update5.15

Report Version: 2021.03.26 05/03/2022 19:47



DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)
Design Methodology: LSD
Service Condition: Dry
LL Deflection Limit: L/360,
TL Deflection Limit: L/240,

Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 1'- 1 1/2"

Factored Resistance of Support Material:

- 769 psi Beam @ 0'- 1 3/4"
- 615 psi Wall @ 9'- 6 1/4"

ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	5'- 1/2"	1.25D + 1.5L	0.88	2621 lb ft	4927 lb ft	Passed - 53%
Factored Shear:	0'- 2 13/16"	1.25D + 1.5L	0.88	1156 lb	1978 lb	Passed - 58%
Live Load (LL) Pos. Defl.:	4'- 11 1/8"	L		0.055"	L/360	Passed - L/999
Total Load (TL) Pos. Defl.:	4'- 10 5/8"	D + L		0.155"	L/240	Passed - L/711

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	2-12	1.25D + 1.5L	0.88	1161 lb		1845 lb	4669 lb	Passed - 63%
2	2-06	1.25D + 1.5L	0.88	1130 lb		1806 lb	3225 lb	Passed - 63%

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	9'- 7 5/8"	Self Weight	Top	3 lb/ft	-	-	-
Uniform	0'- 1 1/2"	2'- 4 1/2"	FC2 Floor Decking (Plan View Fill)	Top	-	4 lb/ft	-	-
Uniform	0'- 1 1/2"	1'- 2 7/8"	FC2 Floor Decking (Plan View Fill)	Top	-	15 lb/ft	-	-
Uniform	0'- 10 1/4"	7'- 1 3/4"	18(i51777)	Top	68 lb/ft	-	-	-
Uniform	7'- 1 3/4"	9'- 7 5/8"	FC2 Floor Decking (Plan View Fill)	Top	9 lb/ft	24 lb/ft	-	-
Tapered	3'- 1/2"	8'- 4 1/2"	Smoothed Load	Front	18 To 16 lb/ft	48 To 42 lb/ft	-	-
Point	1'- 2 9/16"	1'- 2 9/16"	-	Front	215 lb	100 lb	-	-
Point	2'- 4 1/2"	2'- 4 1/2"	J5(i55176)	Front	21 lb	55 lb	-	-
Point	9'- 1/2"	9'- 1/2"	J5(i55181)	Front	14 lb	39 lb	-	-
Point	7'- 3 1/8"	7'- 3 1/8"	-	Back	248 lb	104 lb	-	-

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 2 3/4"	ST. BEAM (DR.)(i41671)	583 lb	289 lb	-	-
2	9'- 5 1/4"	9'- 7 5/8"	W41(i51905)	495 lb	340 lb	-	-

DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.

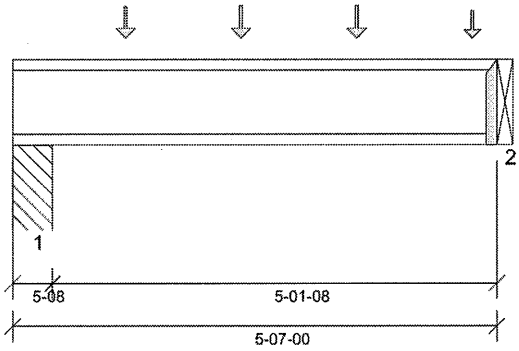
S3046091

MiTek	Customer: **Gold Park Homes** Job Address: **Pine Valley Ph2** City: **Vaughan** Job Track: **45147**	Job Name: **343077 Ground A + Second A...** Level: **Ground Floor** Label: **B35 - i54409** Type: **Beam**	**1 Ply Member** **11 7/8" NI-20**	Status: **Design Passed**

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in MiTek® Structure Version 8.5.3.233.Update5.15

Report Version: 2021.03.26 05/03/2022 19:47



DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)
Design Methodology: LSD
Service Condition: Dry
LL Deflection Limit: L/360,
TL Deflection Limit: L/240,

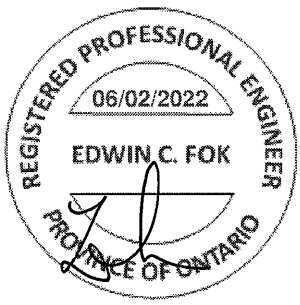
Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 1'- 1 1/2"

Factored Resistance of Support Material:

- 1334 psi Column @ 0'- 4 1/2"
- 769 psi Beam @ 5'- 7"



ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	2'- 7 5/8"	1.25D + 1.5L	0.90	614 lb ft	5002 lb ft	Passed - 12%
Factored Shear:	5'- 6 15/16"	1.25D + 1.5L	0.90	469 lb	2008 lb	Passed - 23%
Total Load (TL) Pos. Defl.:	2'- 11 5/8"	D + L		0.016"	L/240	Passed - L/999

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	5-08	1.25D + 1.5L	0.90	410 lb		2008 lb	16447 lb	Passed - 20%
2	1-12	1.25D + 1.5L	0.90	469 lb		1970 lb	-	Passed - 24%

CONNECTOR INFORMATION

ID	Part No.	Manufacturer	Nailing Requirements			Other Information or Requirement for Reinforcement Accessories
			Top	Face	Member	
2	LT251188		-	-	-	Connector manually specified by the user.

* Connectors: Refer to manufacturer's specifications, fasteners requirements and installation instruction. Where header fasteners are longer than the width of the supporting member, install backer block or clinch header nails.

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	5'- 7"	Self Weight	Top	3 lb/ft	-	-	-
Point	1'- 3 5/8"	1'- 3 5/8"	J5(i54973)	Front	103 lb	64 lb	-	-
Point	2'- 7 5/8"	2'- 7 5/8"	J5(i54974)	Front	106 lb	68 lb	-	-
Point	3'- 11 5/8"	3'- 11 5/8"	J5(i54975)	Front	106 lb	68 lb	-	-
Point	5'- 3 5/8"	5'- 3 5/8"	J5(i54976)	Front	72 lb	50 lb	-	-

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 5 1/2"	Pt1(i54293)	186 lb	112 lb	-	-
2	5'- 7"	5'- 7"	B36(i54820)	217 lb	138 lb	-	-

DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.

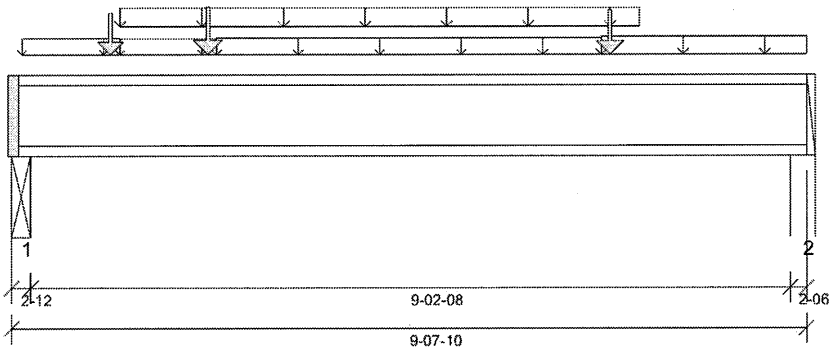
S6046692

MiTek	Customer: **Gold Park Homes** Job Address: **Pine Valley Ph2** City: **Vaughan** Job Track: **45147**	Job Name: **343077 Ground A + Second A...** Level: **Ground Floor** Label: **B36 - i54820** Type: **Beam**	**2 Ply Member** **11 7/8" NI-20**	Status: **Design Passed**

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in MiTek® Structure Version 8.5.3.233.Update5.15

Report Version: 2021.03.26 05/03/2022 19:47



DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)
Design Methodology: LSD
Service Condition: Dry
LL Deflection Limit: L/360,
TL Deflection Limit: L/240,

Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 4'- 8"

Factored Resistance of Support Material:

- 769 psi Beam @ 0'- 1 3/4"
- 615 psi Wall @ 9'- 6 1/4"

ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	4'- 5 1/4"	1.25D + 1.5L	0.81	2576 lb ft	9086 lb ft	Passed - 28%
Factored Shear:	0'- 2 13/16"	1.25D + 1.5L	0.81	1295 lb	3647 lb	Passed - 36%
Live Load (LL) Pos. Defl.:	4'- 9 7/16"	L		0.022"	L/360	Passed - L/999
Total Load (TL) Pos. Defl.:	4'- 9 3/16"	D + L		0.079"	L/240	Passed - L/999

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	2-12	1.25D + 1.5L	0.81	1301 lb		3403 lb	8610 lb	Passed - 38%
2	2-06	1.25D + 1.5L	0.81	1025 lb		3330 lb	5948 lb	Passed - 31%

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	9'- 7 5/8"	Self Weight	Top	6 lb/ft	-	-	-
Uniform	0'- 1 1/2"	1'- 3 3/4"	FC2 Floor Decking (Plan View Fill)	Top	-	18 lb/ft	-	-
Uniform	1'- 3 3/4"	7'- 7 1/4"	16(i51775)	Top	68 lb/ft	-	-	-
Uniform	1'- 3 3/4"	2'- 5 3/4"	FC2 Floor Decking (Plan View Fill)	Top	-	14 lb/ft	-	-
Uniform	2'- 5 3/4"	7'- 1 3/4"	FC2 Floor Decking (Plan View Fill)	Top	7 lb/ft	18 lb/ft	-	-
Uniform	7'- 1 3/4"	9'- 7 5/8"	FC2 Floor Decking (Plan View Fill)	Top	15 lb/ft	39 lb/ft	-	-
Point	1'- 2 3/8"	1'- 2 3/8"	-	Front	228 lb	60 lb	-	-
Point	7'- 3"	7'- 3"	B32(i54376)	Front	216 lb	102 lb	-	-
Point	2'- 4 1/2"	2'- 4 1/2"	B35(i54409)	Back	217 lb	138 lb	-	-

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 2 3/4"	ST. BEAM (DR.)(i41671)	712 lb	270 lb	-	-
2	9'- 5 1/4"	9'- 7 5/8"	W41(i51905)	520 lb	253 lb	-	-

DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.

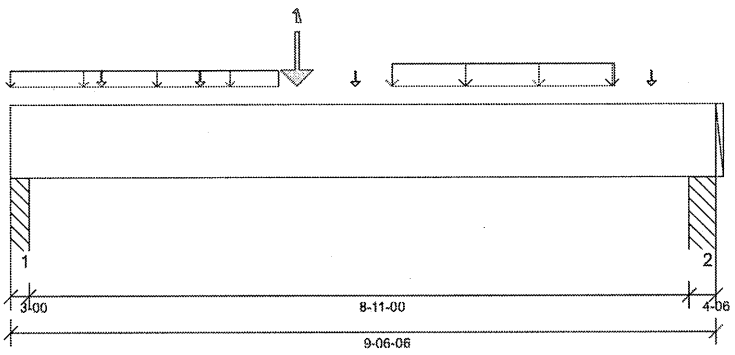
PLY TO PLY CONNECTION

- Member design assumed proper ply to ply connection by others. Verify connection between plies according to code specification and follow the manufacturer's installation instruction. Loads assumed to be distributed equally to each ply.

SEQ46693

MiTek	Customer: **Gold Park Homes** Job Address: **Pine Valley Ph2** City: **Vaughan** Job Track: **45147**	Job Name: **343077 Ground A + Second A...** Level: **Ground Floor** Label: **B37 (-3R) - i54321** Type: **Beam**	**2 Ply Member** **1 3/4" x 11 7/8" 1.55E TimberStrand® LSL**	Status: **Design Passed**

Illustration Not to Scale. Pitch: 0/12 — Designed by Single Member Design Engine in MiTek® Structure Version 8.5.3.233.Update5.15 — Report Version: 2021.03.26 05/03/2022 19:48



DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)
Design Methodology: LSD
Service Condition: Dry
LL Deflection Limit: L/360,
TL Deflection Limit: L/240,

Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 1'- 1 1/2"

Factored Resistance of Support Material:

- 1334 psi Column @ 0'- 2"
- 1334 psi Column @ 9'- 3"

NAIL ONE PLY TO ANOTHER WITH 3-1/2" SPIRAL NAILS @ 12" O/C STAGGERED IN 2 ROWS

ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	3'- 10 1/2"	1.25D + 1.5L	1.00	14921 lb ft	26531 lb ft	Passed - 56%
Factored Shear:	1'- 2 7/8"	1.25D + 1.5L	1.00	4458 lb	14414 lb	Passed - 31%
Live Load (LL) Pos. Defl.:	4'- 6 3/16"	L		0.108"	L/360	Passed - L/993
Total Load (TL) Pos. Defl.:	4'- 6 3/16"	D + L		0.181"	L/240	Passed - L/592

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	3-00	1.25D + 1.5L	1.00	4587 lb		13759 lb	14011 lb	Passed - 33%
2	4-06	1.25D + 1.5L	1.00	3496 lb		20065 lb	20432 lb	Passed - 17%

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	9'- 6 3/8"	Self Weight	Top	13 lb/ft	-	-	-
Uniform	0'	3'- 7 1/2"	User Load	Top	13 lb/ft	34 lb/ft	-	-
Uniform	5'- 1 15/16"	8'- 1 15/16"	Smoothed Load	Back	64 lb/ft	128 lb/ft	-	-
Point	1'- 2 3/4"	1'- 2 3/4"	J3(i54701)	Back	67 lb	178 lb	-	-
Point	2'- 6 3/4"	2'- 6 3/4"	J3(i54792)	Back	64 lb	170 lb	-	-
Point	3'- 10 1/2"	3'- 10 1/2"	B18 (-3R)(i54371)	Back	1688 lb	2376/-35 lb	-	-
Point	4'- 7 15/16"	4'- 7 15/16"	J3(i54929)	Back	57 lb	115 lb	-	-
Point	8'- 7 15/16"	8'- 7 15/16"	J3(i54829)	Back	56 lb	113 lb	-	-

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 3"	Pt2(i54691)	1302 lb	1973/-21 lb	-	-
2	9'- 2"	9'- 6 3/8"	Pt2(i54221)	1003 lb	1495/-14 lb	-	-

DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Lateral stability factor (KL) was based on user preference to use the width of one ply.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.

PLY TO PLY CONNECTION

- Member design assumed proper ply to ply connection by others. Fastener spacing along length of member must not exceed 4 times depth of member. Verify connection between plies according to code specification and follow the manufacturer's installation instruction. Loads assumed to be distributed equally to each ply.

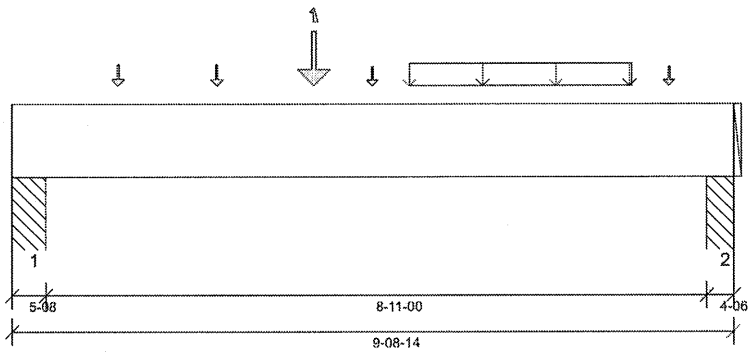
S30466814

MiTek	Customer: **Gold Park Homes** Job Address: **Pine Valley Ph2** City: **Vaughan** Job Track: **45147**	Job Name: **343077 Ground A + Second A...** Level: **Ground Floor** Label: **B38 (-3R) - i54892** Type: **Beam**	**2 Ply Member** **1 3/4" x 11 7/8" 1.55E TimberStrand® LSL**	Status: **Design Passed**

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in MiTek® Structure Version 8.5.3.233.Update5.15

Report Version: 2021.03.26 05/03/2022 19:48



DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)
Design Methodology: LSD
Service Condition: Dry
LL Deflection Limit: L/360,
TL Deflection Limit: L/240,

Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 1'- 1 1/2"

Factored Resistance of Support Material:

- 1334 psi Column @ 0'- 4 1/2"
- 1334 psi Column @ 9'- 5 1/2"

NAIL ONE PLY TO ANOTHER WITH 3-1/2" SPIRAL NAILS @ 12" O/C STAGGERED IN 2 ROWS

REGISTERED PROFESSIONAL ENGINEER
06/02/2022
EDWIN C. FOK
PROVINCE OF ONTARIO

ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	4'- 1"	1.25D + 1.5L	1.00	6165 lb ft	26531 lb ft	Passed - 23%
Factored Shear:	1'- 5 3/8"	1.25D + 1.5L	1.00	2044 lb	14414 lb	Passed - 14%
Live Load (LL) Pos. Defl.:	4'- 9 11/16"	L		0.046"	L/360	Passed - L/999
Total Load (TL) Pos. Defl.:	4'- 9 9/16"	D + L		0.080"	L/240	Passed - L/999

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	5-08	1.25D + 1.5L	1.00	2068 lb		25225 lb	25687 lb	Passed - 8%
2	4-06	1.25D + 1.5L	1.00	1867 lb		20065 lb	20432 lb	Passed - 9%

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	9'- 8 7/8"	Self Weight	Top	13 lb/ft	-	-	-
Uniform	5'- 4 7/16"	8'- 4 7/16"	Smoothed Load	Front	64 lb/ft	128 lb/ft	-	-
Point	1'- 5 1/4"	1'- 5 1/4"	J3(i54701)	Front	67 lb	178 lb	-	-
Point	2'- 9 1/4"	2'- 9 1/4"	J3(i54792)	Front	64 lb	170 lb	-	-
Point	4'- 1"	4'- 1"	B18 (-3R)(i54371)	Front	614 lb	683/-9 lb	-	-
Point	4'- 10 7/16"	4'- 10 7/16"	J3(i54929)	Front	57 lb	115 lb	-	-
Point	8'- 10 7/16"	8'- 10 7/16"	J3(i54829)	Front	56 lb	113 lb	-	-

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 5 1/2"	Pt2(i54771)	621 lb	861/-5 lb	-	-
2	9'- 4 1/2"	9'- 8 7/8"	Pt2(i54318)	556 lb	782/-4 lb	-	-

DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Lateral stability factor (KL) was based on user preference to use the width of one ply.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.

PLY TO PLY CONNECTION

- Member design assumed proper ply to ply connection by others. Fastener spacing along length of member must not exceed 4 times depth of member. Verify connection between plies according to code specification and follow the manufacturer's installation instruction. Loads assumed to be distributed equally to each ply.

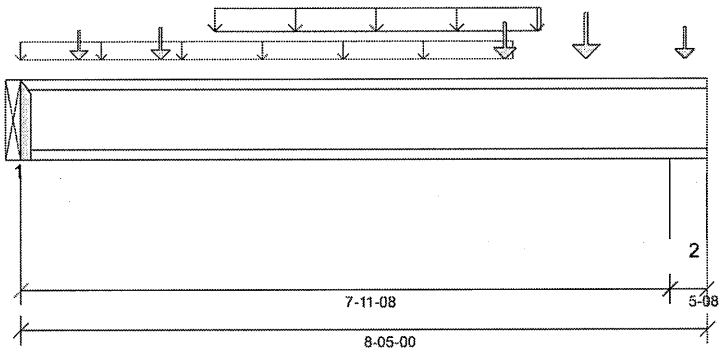
87046695

MiTek	Customer: **Gold Park Homes** Job Address: **Pine Valley Ph2** City: **Vaughan** Job Track: **45147**	Job Name: **343077 Second A W Elevator WD** Level: **Second Floor** Label: **B39 - i55302** Type: **Beam**	**1 Ply Member** **11 7/8" NI-20**	Status: **Design Passed**

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in MiTek® Structure Version 8.5.3.233.Update5.15

Report Version: 2021.03.26 05/04/2022 11:16



DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)
Design Methodology: LSD
Service Condition: Dry
LL Deflection Limit: L/360,
TL Deflection Limit: L/240,

Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 1'- 1 1/2"

Factored Resistance of Support Material:

- 769 psi Beam @ 0'
- 615 psi Wall @ 8'- 1/2"

REGISTERED PROFESSIONAL ENGINEER
06/02/2022
EDWIN C. FOK
PROVINCE OF ONTARIO

ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	4'- 4 5/8"	1.25D + 1.5L	1.00	2925 lb ft	5580 lb ft	Passed - 52%
Factored Shear:	7'- 11 7/16"	1.25D + 1.5L	1.00	1436 lb	2240 lb	Passed - 64%
Live Load (LL) Pos. Defl.:	4'- 15/16"	L		0.066"	L/360	Passed - L/999
Total Load (TL) Pos. Defl.:	4'- 1 1/16"	D + L		0.130"	L/240	Passed - L/732

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	1-12	1.25D + 1.5L	1.00	1290 lb		1970 lb	-	Passed - 65%
2	5-08	1.25D + 1.5L	1.00	1702 lb		2240 lb	8459 lb	Passed - 76%

CONNECTOR INFORMATION

ID	Part No.	Manufacturer	Nailing Requirements			Other Information or Requirement for Reinforcement Accessories
			Top	Face	Member	
1	LT251188		-	-	-	Connector manually specified by the user.

* Connectors: Refer to manufacturer's specifications, fasteners requirements and installation instruction. Where header fasteners are longer than the width of the supporting member, install backer block or clinch header nails.

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	8'- 5"	Self Weight	Top	3 lb/ft	-	-	-
Uniform	0'	6'- 1/2"	20(i51780)	Top	61 lb/ft	-	-	-
Uniform	2'- 4 5/8"	6'- 4 5/8"	Smoothed Load	Back	44 lb/ft	118 lb/ft	-	-
Point	5'- 11 5/16"	5'- 11 5/16"	-	Front	191 lb	49 lb	-	-
Point	6'- 11 1/4"	6'- 11 1/4"	-	Front	94 lb	251 lb	-	-
Point	8'- 1 3/4"	8'- 1 3/4"	J7(i55666)	Front	50 lb	134 lb	-	-
Point	0'- 8 5/8"	0'- 8 5/8"	J6(i57067)	Back	43 lb	115 lb	-	-
Point	1'- 8 5/8"	1'- 8 5/8"	J6(i56330)	Back	52 lb	138 lb	-	-

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'	B30(i55779)	465 lb	471 lb	-	-
2	7'- 11 1/2"	8'- 5"	25(i51910)	533 lb	692 lb	-	-

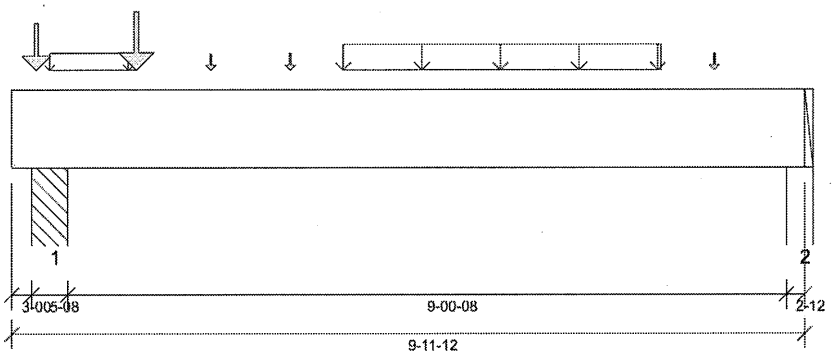
DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.

S6046696

MiTek	Customer: **Gold Park Homes** Job Address: **Pine Valley Ph2** City: **Vaughan** Job Track: **45147**	Job Name: **343077 Second A W Elevator WD** Level: **Second Floor** Label: **B40 - i56220** Type: **Beam**	**2 Ply Member** **1 3/4" x 11 7/8" 1.55E TimberStrand® LSL**	Status: **Design Passed**

Illustration Not to Scale. Pitch: 0/12 — Designed by Single Member Design Engine in MiTek® Structure Version 8.5.3.233.Update5.15 — Report Version: 2021.03.26 05/04/2022 11:17



DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)
Design Methodology: LSD
Service Condition: Dry
LL Deflection Limit: L/360,
TL Deflection Limit: L/240,

Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 1'- 1 1/2"

Factored Resistance of Support Material:

- 1334 psi Column @ 0'- 5 3/4"
- 615 psi Wall @ 9'- 10"

NAIL ONE PLY TO ANOTHER WITH 3-1/2" SPIRAL NAILS @ 12" O/C STAGGERED IN 2 ROWS

REGISTERED PROFESSIONAL ENGINEER
06/02/2022
EDWIN C. FOK
PROVINCE OF ONTARIO

ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	2'- 6 1/8"	1.25D + 1.5L	1.00	7777 lb ft	26531 lb ft	Passed - 29%
Factored Neg. Moment:	0'- 5 3/4"	1.25D + 1.5L	1.00	914 lb ft	25803 lb ft	Passed - 4%
Factored Shear:	1'- 8 3/8"	1.25D + 1.5L	1.00	7628 lb	14414 lb	Passed - 53%
Live Load (LL) Pos. Defl.:	4'- 9 3/8"	L		0.064"	L/360	Passed - L/999
Total Load (TL) Pos. Defl.:	4'- 9 11/16"	D + L		0.118"	L/240	Passed - L/923

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	5-08	1.25D + 1.5L	1.00	13211 lb		25225 lb	25687 lb	Passed - 52%
2	2-12	1.25D + 1.5L	1.00	2144 lb		12613 lb	5921 lb	Passed - 36%

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	9'- 11 3/4"	Self Weight	Top	13 lb/ft	-	-	-
Uniform	0'- 5 3/4"	1'- 6 7/8"	FC3 Floor Decking (Plan View Fill)	Top	47 lb/ft	-	-	-
Uniform	4'- 2 1/8"	8'- 2 1/8"	Smoothed Load	Front	106 lb/ft	122 lb/ft	-	-
Point	1'- 6 7/8"	1'- 6 7/8"	B30(i55779)	Front	2098 lb	3131 lb	-	-
Point	2'- 6 1/8"	2'- 6 1/8"	J6(i57067)	Front	102 lb	118 lb	-	-
Point	3'- 6 1/8"	3'- 6 1/8"	J6(i56330)	Front	123 lb	142 lb	-	-
Point	8'- 10 1/8"	8'- 10 1/8"	J6(i56332)	Front	107 lb	139 lb	-	-
Point	0'- 3 3/4"	0'- 3 3/4"	B22(i56221)	Back	1152 lb	2669 lb	-	-

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'- 3"	0'- 8 1/2"	Pt1(i55224)	3470 lb	5851 lb	-	-
2	9'- 9"	9'- 11 3/4"	9(i41702)	757 lb	864 lb	-	-

DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Lateral stability factor (KL) was based on user preference to use the width of one ply.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- The deflection at the cantilever for either live and/or total loads is less than 3/8" and therefore has been excluded from the deflection ratio considerations.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.
- Bearing capacity of member at support 1, 2 was verified for the effect of concentrated load applied near the support. At support 1. Required Load Area: L=3.500", W=3.500". LDF=1.00, Pf=5444 lb, Q'r=13759 lb, Result=39.56%.

PLY TO PLY CONNECTION

- Member design assumed proper ply to ply connection by others. Fastener spacing along length of member must not exceed 4 times depth of member. Verify connection between plies according to code specification and follow the manufacturer's installation instruction. Loads assumed to be distributed equally to each ply.

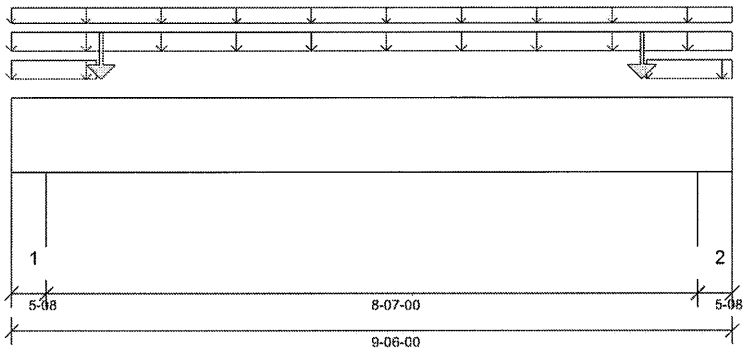
SG046697

MiTek	Customer: **Gold Park Homes** Job Address: **Pine Valley Ph2** City: **Vaughan** Job Track: **45147**	Job Name: **343077 Ground B + Second B (3,** Level: **Second Floor** Label: **B41 - i51809** Type: **Beam**	**2 Ply Member** **1 3/4" x 11 7/8" 1.55E TimberStrand® LSL**	Status: **Design Passed**

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in MiTek® Structure Version 8.5.3.233.Update5.15

Report Version: 2021.03.26 05/05/2022 13:45



DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)
Design Methodology: LSD
Service Condition: Dry
LL Deflection Limit: L/360,
TL Deflection Limit: L/240,

Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 8'- 7"

Factored Resistance of Support Material:

- 615 psi Wall @ 0'- 4 1/2"
- 615 psi Wall @ 9'- 1 1/2"

NAIL ONE PLY TO ANOTHER WITH 3-1/2" SPIRAL NAILS @12" O/C STAGGERED IN 2 ROWS

ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	4'- 9"	1.4D	0.65	1828 lb ft	17245 lb ft	Passed - 11%
Factored Shear:	8'- 5/8"	1.25D + 1.5L	0.86	1020 lb	12332 lb	Passed - 8%
Total Load (TL) Pos. Defl.:	4'- 9"	D + L		0.033"	L/240	Passed - L/999

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	5-08	1.25D + 1.5L	0.86	1599 lb		21583 lb	10133 lb	Passed - 16%
2	5-08	1.25D + 1.5L	0.86	1599 lb		21583 lb	10133 lb	Passed - 16%

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	9'- 6"	Self Weight	Top	13 lb/ft	-	-	-
Uniform	0'	9'- 6"	E19(i41624)	Top	101 lb/ft	-	-	-
Uniform	0'	9'- 6"	FC3 Floor Decking (Plan View Fill)	Top	8 lb/ft	23 lb/ft	-	-
Uniform	0'	1'- 1 1/2"	E19(i41624)	Top	41 lb/ft	63 lb/ft	-	-
Uniform	8'- 4 1/2"	9'- 6"	E19(i41624)	Top	41 lb/ft	63 lb/ft	-	-
Point	1'- 2 1/4"	1'- 2 1/4"	E19(i41624)	Top	162 lb	228 lb	-	-
Point	8'- 3 3/4"	8'- 3 3/4"	E19(i41624)	Top	162 lb	228 lb	-	-

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 5 1/2"	E33(i51784)	790 lb	405 lb	-	-
2	9'- 1/2"	9'- 6"	E31(i51783)	792 lb	409 lb	-	-

DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Lateral stability factor (KL) was based on user preference to use the width of one ply.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.

PLY TO PLY CONNECTION

- Member design assumed proper ply to ply connection by others. Fastener spacing along length of member must not exceed 4 times depth of member. Verify connection between plies according to code specification and follow the manufacturer's installation instruction. Loads assumed to be distributed equally to each ply.

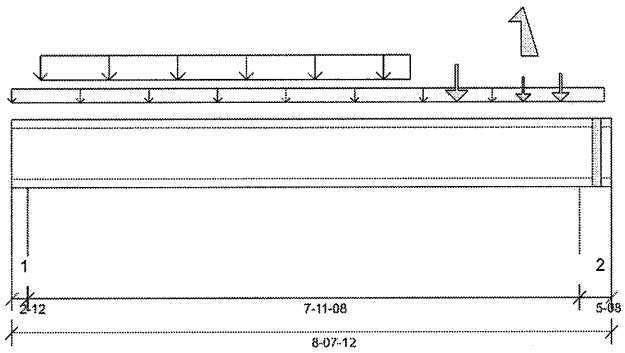
C6046088

MiTek	Customer: **Gold Park Homes** Job Address: **Pine Valley Ph2** City: **Vaughan** Job Track: **45147**	Job Name: **343077 Ground C + Second C (5,** Level: **Second Floor** Label: **B42 - i54253** Type: **Beam**	**2 Ply Member** **11 7/8" NI-20**	Status: **Design Passed**

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in MiTek® Structure Version 8.5.3.233.Update5.15

Report Version: 2021.03.26 05/06/2022 16:53



DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)
Design Methodology: LSD
Service Condition: Dry
LL Deflection Limit: L/360,
TL Deflection Limit: L/240,

Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 1'- 1 1/2"

Factored Resistance of Support Material:

- 615 psi Wall @ 0'- 1 3/4"
- 615 psi Wall @ 8'- 3 1/4"

ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	3'- 9"	1.25D + 1.5L	1.00	3568 lb ft	11160 lb ft	Passed - 32%
Factored Neg. Moment:	7'- 4 1/2"	0.9D + 1.5L + S	0.85	228 lb ft	9531 lb ft	Passed - 2%
Factored Shear:	8'- 2 3/16"	1.25D + 1.5L	1.00	1669 lb	4480 lb	Passed - 37%
Live Load (LL) Pos. Defl.:	4'- 2 7/16"	L		0.057"	L/360	Passed - L/999
Total Load (TL) Pos. Defl.:	4'- 2 1/4"	D + L		0.077"	L/240	Passed - L/999

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	2-12	1.25D + 1.5L	1.00	1661 lb		4180 lb	8459 lb	Passed - 40%
2	5-08	1.25D + 1.5L + S	1.00	1531 lb		4480 lb	16918 lb	Passed - 34%
2	5-08	0.9D + 1.5L + S	0.85		-199 lb	-	-	

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	8'- 7 3/4"	Self Weight	Top	6 lb/ft	-	-	-
Uniform	0'	8'- 6 1/2"	FC3 Floor Decking (Plan View Fill)	Top	7 lb/ft	19 lb/ft	-	-
Uniform	0'- 5"	5'- 9"	Smoothed Load	Front	75 lb/ft	201 lb/ft	-	-
Point	6'- 5"	6'- 5"	J4(i54292)	Front	86 lb	230 lb	-	-
Point	7'- 4 1/2"	7'- 4 1/2"	APP (CANT.) (i54354)	Front	-80 lb	153/-219 lb	-175 lb	-
Point	7'- 11"	7'- 11"	J4(i54311)	Front	83 lb	116 lb	-	-

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 2 3/4"	23(i51824)	325 lb	840/-24 lb	-19 lb	-
2	8'- 2 1/4"	8'- 7 3/4"	4(i41664)	275 lb	891/-195 lb	-156 lb	-

DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.

PLY TO PLY CONNECTION

- Member design assumed proper ply to ply connection by others. Verify connection between plies according to code specification and follow the manufacturer's installation instruction. Loads assumed to be distributed equally to each ply.

REGISTERED PROFESSIONAL ENGINEER
06/02/2022
EDWIN C. FOK
PROVINCE OF ONTARIO

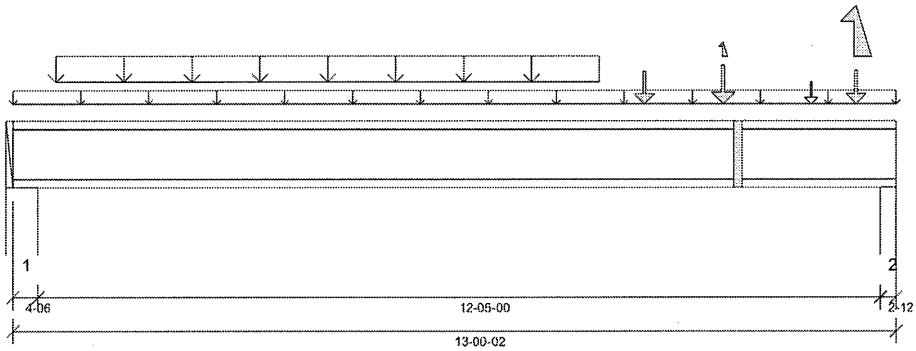
S6046699

MiTek	Customer: **Gold Park Homes** Job Address: **Pine Valley Ph2** City: **Vaughan** Job Track: **45147**	Job Name: **343077 Ground C + Second C (5,** Level: **Second Floor** Label: **B43 - i54260** Type: **Beam**	**2 Ply Member** **11 7/8" NI-20**	Status: **Design Passed**

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in MiTek® Structure Version 8.5.3.233.Update5.15

Report Version: 2021.03.26 05/06/2022 16:53



DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)
Design Methodology: LSD
Service Condition: Dry
LL Deflection Limit: L/360,
TL Deflection Limit: L/240,

Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 1'- 1 1/2"

Factored Resistance of Support Material:

- 615 psi Wall @ 0'- 3 3/8"
- 615 psi Wall @ 12'- 10 3/8"

REGISTERED PROFESSIONAL ENGINEER
06/02/2022
EDWIN C. FOK
PROVINCE OF ONTARIO

ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	6'- 7 5/8"	1.25D + 1.5L	1.00	9712 lb ft	11160 lb ft	Passed - 87%
Factored Neg. Moment:	12'- 3 1/8"	0.9D + 1.5L + S	0.79	165 lb ft	8764 lb ft	Passed - 2%
Factored Shear:	0'- 4 7/16"	1.25D + 1.5L	1.00	2910 lb	4480 lb	Passed - 65%
Live Load (LL) Pos. Defl.:	6'- 6 13/16"	L		0.313"	L/360	Passed - L/476
Total Load (TL) Pos. Defl.:	6'- 6 5/8"	D + L		0.431"	L/240	Passed - L/345

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	4-06	1.25D + 1.5L	1.00	2929 lb		4480 lb	13457 lb	Passed - 65%
2	2-12	1.25D + 1.5L + S	1.00	2691 lb		4180 lb	8459 lb	Passed - 64%
2	2-12	0.9D + 1.5L + S	0.79		-212 lb	-	-	

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	13'- 1/8"	Self Weight	Top	6 lb/ft	-	-	-
Uniform	0'	13'- 1/8"	FC3 Floor Decking (Plan View Fill)	Top	7 lb/ft	19 lb/ft	-	-
Uniform	0'- 7 5/8"	8'- 7 5/8"	Smoothed Load	Front	86 lb/ft	229 lb/ft	-	-
Point	9'- 3 5/8"	9'- 3 5/8"	J2(i54041)	Front	97 lb	260 lb	-	-
Point	10'- 5 1/2"	10'- 5 1/2"	-	Front	130 lb	334/-20 lb	-14 lb	-
Point	11'- 9 1/8"	11'- 9 1/8"	J2(i54013)	Front	44 lb	119 lb	-	-
Point	12'- 5 1/16"	12'- 5 1/16"	-	Front	74/-257 lb	301/-228 lb	-188 lb	-

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 4 3/8"	E12(i41614)	573 lb	1470/-15 lb	-12 lb	-
2	12'- 9 3/8"	13'- 1/8"	23(i51824)	365 lb	1622/-233 lb	-190 lb	-

DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.

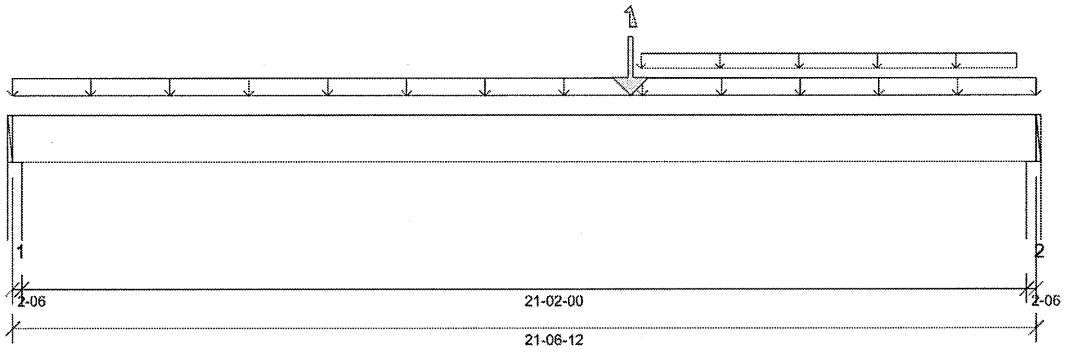
PLY TO PLY CONNECTION

- Member design assumed proper ply to ply connection by others. Verify connection between plies according to code specification and follow the manufacturer's installation instruction. Loads assumed to be distributed equally to each ply.

SE046700

MiTek	Customer: **Gold Park Homes** Job Address: **Pine Valley Ph2** City: **Vaughan** Job Track: **45147**	Job Name: **343077 Ground C + Second C (5,** Level: **Ground Floor** Label: **B44 - i54757** Type: **Beam**	**4 Ply Member** **1 3/4" x 11 7/8" 1.55E TimberStrand® LSL**	Status: **Design Passed**

Illustration Not to Scale. Pitch: 0/12 — Designed by Single Member Design Engine in MiTek® Structure Version 8.5.3.233.Update5.15 — Report Version: 2021.03.26 05/06/2022 16:54



DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)
Design Methodology: LSD
Service Condition: Dry
LL Deflection Limit: L/360,
TL Deflection Limit: L/240,

Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:
Top: 0' Bottom: 21'- 2"

Factored Resistance of Support Material:

- 615 psi Wall @ 0'- 1 3/8"
- 615 psi Wall @ 21'- 5 3/8"

CONNECT 4 PLY MEMBERS WITH SIMPSONS SDW22614 WOOD SCREWS @ 24" O.C., STAGGERED IN 2 ROWS (TOP LOADED)

ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	13'- 1/8"	1.25D + 1.5L	1.00	29608 lb ft	53063 lb ft	Passed - 56%
Factored Shear:	20'- 4 1/2"	1.25D + 1.5L	1.00	3879 lb	28828 lb	Passed - 13%
Live Load (LL) Pos. Defl.:	11'- 3 13/16"	L		0.679"	L/360	Passed - L/374
Total Load (TL) Pos. Defl.:	11'- 3 1/4"	D + L		0.975"	L/240	Passed - L/260
Permanent Deflection:	11'- 2"			-	L/360	Passed - L/883

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	2-06	1.25D + 1.5L	1.00	3033 lb		21785 lb	10228 lb	Passed - 30%
2	2-06	1.25D + 1.5L	1.00	4023 lb		21785 lb	10228 lb	Passed - 39%

SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	21'- 6 3/4"	Self Weight	Top	26 lb/ft	-	-	-
Uniform	0'	21'- 6 3/4"	FC2 Floor Decking (Plan View Fill)	Top	15 lb/ft	40 lb/ft	-	-
Uniform	13'- 2 7/8"	21'- 1 7/8"	FC2 Floor Decking (Plan View Fill)	Top	5 lb/ft	-	-	-
Point	13'- 1/8"	13'- 1/8"	23(i51824)	Top	721 lb	2462/-257 lb	-209 lb	-

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 2 3/8"	W12(i41590)	728 lb	1382/-99 lb	-80 lb	-
2	21'- 4 3/8"	21'- 6 3/4"	W17(i41595)	919 lb	1950/-158 lb	-129 lb	-

DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Lateral stability factor (KL) was based on user preference to use the width of one ply.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.

PLY TO PLY CONNECTION

- Member design assumed proper ply to ply connection by others. Fastener spacing along length of member must not exceed 4 times depth of member. Verify connection between plies according to code specification and follow the manufacturer's installation instruction. Loads assumed to be distributed equally to each ply.

NORDIC
STRUCTURES

Maximum Floor Spans – M4.1, L/360

Design Criteria

Spans:	Simple span
Loads:	Live load = 40 psf and dead load = 20 psf
Deflection limits:	L/360 under live load and L/240 under total load
Sheathing:	3/4 in. nailed-glued oriented strand board (OSB) sheathing

Maximum Floor Spans

Joist depth	Joist series	Bare, On centre spacing				1/2 in. gypsum ceiling, On centre spacing			
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
9-1/2"	NI-20	15'-11"	15'-0"	14'-6"	13'-5"	16'-5"	15'-5"	14'-6"	13'-5"
	NI-40x	17'-0"	16'-0"	15'-5"	14'-10"	17'-5"	16'-5"	15'-10"	14'-11"
	NI-60	17'-2"	16'-2"	15'-7"	14'-11"	17'-7"	16'-7"	16'-0"	15'-4"
	NI-80	18'-3"	17'-1"	16'-5"	15'-9"	18'-8"	17'-5"	16'-9"	16'-1"
11-7/8"	NI-20	17'-11"	16'-11"	16'-3"	15'-8"	18'-7"	17'-5"	16'-10"	16'-1"
	NI-40x	19'-4"	17'-11"	17'-3"	16'-7"	19'-11"	18'-6"	17'-9"	17'-0"
	NI-60	19'-7"	18'-2"	17'-6"	16'-9"	20'-2"	18'-9"	17'-11"	17'-2"
	NI-80	21'-1"	19'-6"	18'-6"	17'-7"	21'-7"	20'-0"	19'-0"	18'-0"
	NI-90	21'-6"	19'-10"	18'-11"	17'-11"	22'-0"	20'-4"	19'-5"	18'-4"
14"	NI-40x	21'-5"	19'-11"	18'-11"	18'-0"	22'-1"	20'-7"	19'-7"	18'-7"
	NI-60	21'-10"	20'-2"	19'-3"	18'-3"	22'-6"	20'-10"	19'-11"	18'-10"
	NI-80	23'-5"	21'-7"	20'-7"	19'-5"	24'-0"	22'-3"	21'-2"	20'-0"
	NI-90	23'-10"	22'-1"	21'-0"	19'-10"	24'-5"	22'-7"	21'-6"	20'-4"
16"	NI-60	23'-9"	22'-0"	21'-0"	19'-10"	24'-6"	22'-9"	21'-8"	20'-7"
	NI-80	25'-6"	23'-7"	22'-5"	21'-2"	26'-2"	24'-3"	23'-1"	21'-10"
	NI-90	26'-0"	24'-0"	22'-10"	21'-6"	26'-7"	24'-8"	23'-5"	22'-2"

Joist depth	Joist series	Mid-span blocking with 1x4 inch strap, On centre spacing				Mid-span blocking and 1/2 in. gypsum ceiling, On centre spacing			
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
9-1/2"	NI-20	17'-1"	15'-5"	14'-6"	13'-5"	17'-1"	15'-5"	14'-6"	13'-5"
	NI-40x	18'-8"	17'-6"	16'-7"	14'-11"	19'-2"	17'-8"	16'-7"	14'-11"
	NI-60	18'-11"	17'-8"	16'-10"	15'-7"	19'-5"	18'-0"	16'-10"	15'-7"
	NI-80	20'-3"	18'-10"	17'-11"	17'-2"	20'-8"	19'-3"	18'-4"	17'-5"
11-7/8"	NI-20	20'-3"	18'-8"	17'-6"	16'-1"	20'-7"	18'-8"	17'-6"	16'-1"
	NI-40x	21'-10"	20'-4"	19'-0"	17'-0"	22'-5"	20'-10"	19'-0"	17'-0"
	NI-60	22'-1"	20'-7"	19'-8"	18'-7"	22'-8"	21'-2"	20'-3"	18'-8"
	NI-80	23'-8"	22'-0"	20'-11"	19'-10"	24'-1"	22'-6"	21'-6"	20'-4"
	NI-90	24'-1"	22'-5"	21'-4"	20'-2"	24'-7"	22'-11"	21'-10"	20'-8"
14"	NI-40x	24'-5"	22'-9"	20'-11"	18'-8"	25'-1"	22'-11"	20'-11"	18'-8"
	NI-60	24'-10"	23'-2"	22'-1"	20'-10"	25'-6"	23'-10"	22'-9"	21'-4"
	NI-80	26'-6"	24'-8"	23'-6"	22'-2"	27'-1"	25'-3"	24'-1"	22'-9"
	NI-90	27'-0"	25'-1"	23'-11"	22'-7"	27'-6"	25'-8"	24'-6"	23'-2"
16"	NI-60	27'-3"	25'-5"	24'-3"	22'-11"	28'-0"	26'-2"	25'-0"	23'-1"
	NI-80	29'-1"	27'-1"	25'-9"	24'-4"	29'-8"	27'-9"	26'-5"	25'-0"
	NI-90	29'-7"	27'-6"	26'-2"	24'-9"	30'-2"	28'-2"	26'-10"	25'-5"

Notes:

1. The tabulated clear spans are based on CSA O86-14 and NBC 2015, and are applicable to residential floor construction meeting the above design criteria.
2. For multiple-span applications, the end spans shall be 40% or more of the adjacent span.
3. Minimum bearing length shall be 1-3/4 inch for end bearings, and 3-1/2 inches for intermediate bearings.
4. Bearing stiffeners are not required when I-joists are used in accordance with this table, except as required for hangers.
5. Nordic I-joists are listed in CCMC Evaluation Report 13032-R and APA Product Report PR-L274C.

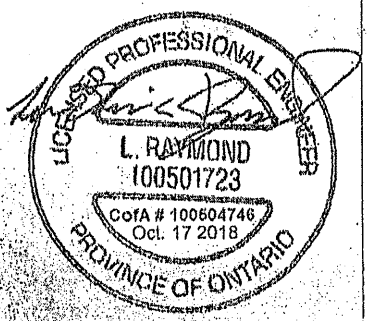
The construction details for residential designs are prone to changes.

Details released after April 2014 supersedes N-C301

Installation must comply with latest documentation on I-Joist and other Nordic products from the http://nordic.ca/

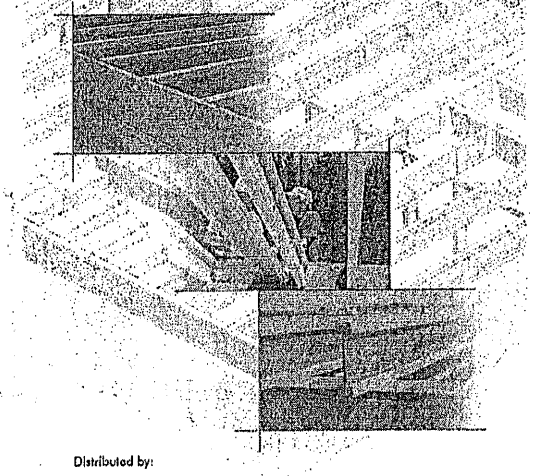
This document does not constitute a record of the structural integrity of the building nor suitability of the design assumptions made. Nordic Structures is responsible only for the structural adequacy of its component based on the design criteria and loadings shown on the calculation sheets.

Document prepared for the use of Stephanie Gon from Alpa, Ontario. (Nordic Request 1810-095)



NORDIC ENGINEERED WOOD

INSTALLATION GUIDE FOR RESIDENTIAL FLOORS



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SAFETY AND CONSTRUCTION PRECAUTIONS

Do not walk on I-Joists until fully fastened and braced, or serious injuries can result.

Never stack building materials over unsheathed I-Joists. Once sheathed, do not over-stress I-Joist with concentrated loads from building materials.

WARNING

I-Joists are not stable until completely installed, and will not carry any load until fully braced and sheathed.

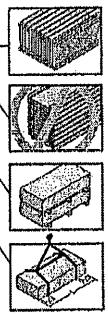
Avoid Accidents by Following these Important Guidelines!

1. Brace and nail each I-Joist as it is installed, using hangers, blocking panels, rim board, and/or cross-bridging at joist ends. When I-Joists are applied continuous over interior supports and a load-bearing wall is planned at that location, blocking will be required at the interior support.
2. When the building is completed, the floor sheathing will provide lateral support for the top flanges of the I-Joists. Until this sheathing is applied, temporary bracing, often called struts, or temporary sheathing must be applied to prevent I-Joist rollover or buckling.
   - Temporary bracing or struts must be 1x4 inch minimum, at least 8 feet long and spaced no more than 8 feet on centre, and must be secured with a minimum of two 2-1/2" nails fastened to the top surface of each I-Joist. Nail the bracing in a lateral restraint at the end of each bay. Lap ends of adjoining bracing over at least two I-Joists.
   - Or, sheathing (temporary or permanent) can be nailed to the top flange of the first 4 feet of I-Joists at the end of the bay.
3. For cantilevered I-Joists, brace top and bottom flanges, and brace ends with closure panels, rim board, or cross-bridging.
4. Install and fully nail permanent sheathing to each I-Joist before placing loads on the floor system. Then, stack building materials over beams or walls only.
5. Never install a damaged I-Joist.

Improper storage or installation, failure to follow applicable building codes, failure to follow span ratings for Nordic I-Joists, failure to follow allowable hole sizes and locations, or failure to use web stiffeners when required can result in serious accidents. Follow these installation guidelines carefully.

STORAGE AND HANDLING GUIDELINES

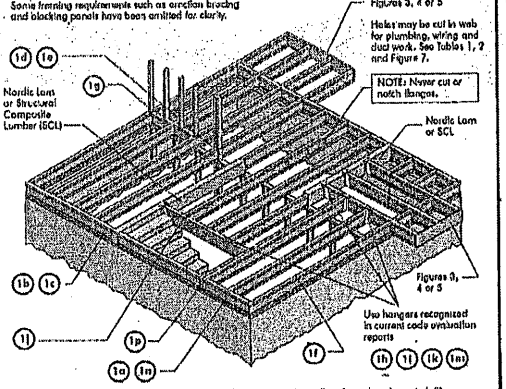
1. Bundle wrap can be slippery when wet. Avoid walking on wrapped bundles.
2. Store, stack, and handle I-Joists vertically and level only.
3. Always stack and handle I-Joists in the upright position only.
4. Do not store I-Joists in direct contact with the ground and/or flatwise.
5. Protect I-Joists from weather, and use spacers to separate bundles.
6. Bundled units should be kept intact until time of installation.
7. When handling I-Joists with a crane on the job site, take a few simple precautions to prevent damage to the I-Joists and injury to your work crew.
   - Pick I-Joists in bundles as shipped by the supplier.
   - Orient the bundles so that the webs of the I-Joists are vertical.
   - Pick the bundles at the 5th points, using a spreader bar if necessary.
8. Do not handle I-Joists in a horizontal orientation.
9. NEVER USE OR TRY TO REPAIR A DAMAGED I-JOIST.



INSTALLING NORDIC I-JOISTS

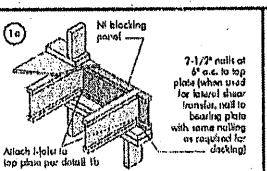
1. Before laying out floor system components, verify that I-Joist flange widths match hanger widths. If not, contact your supplier.
2. Except for cutting to length, I-Joist flanges should never be cut, drilled, or notched.
3. Install I-Joists so that top and bottom flanges are within 1/2 inch of true vertical alignment.
4. I-Joists must be anchored securely to supports before floor sheathing is attached, and supports for multiple-span joists must be level.
5. Minimum bearing lengths: 1-3/4 inches for end bearings and 3-1/2 inches for intermediate bearings.
6. When using hangers, seat I-Joists firmly in hanger bottoms to minimize settlement.
7. Leave a 1/16-inch gap between the I-Joist end and a header.
8. Concentrated loads greater than those that can normally be expected in residential construction should only be applied to the top surface of the top flange. Normal concentrated loads include track lighting fixtures, audio equipment and security cameras. Never suspend unusual or heavy loads from the I-Joist's bottom flange. Whenever possible, suspend all concentrated loads from the top of the I-Joist. Or, attach the load to blocking that has been securely fastened to the I-Joist webs.
9. Never install I-Joists where they will be permanently exposed to weather, or where they will remain in direct contact with concrete or masonry.
10. Restrain ends of floor joists to prevent rollover. Use rim board, rim joists or I-Joist blocking panels.
11. For I-Joists installed over and beneath bearing walls, use full depth blocking panels, rim board, or squash blocks (cripple members) to transfer gravity loads through the floor system to the wall or foundation below.
12. Due to shrinkage, common framing lumber set on edge may never be used as blocking or rim boards. I-Joist blocking panels or other engineered wood products – such as rim board – must be cut to fit between the I-Joists, and an I-Joist-compatible depth selected.
13. Provide permanent lateral support of the bottom flange of all I-Joists at interior supports of multiple-span joists. Similarly, support the bottom flange of all cantilevered I-Joists at the end support next to the cantilever extension. In the completed structure, the gypsum wallboard ceiling provides this lateral support. Until the final finished ceiling is applied, temporary bracing or struts must be used.
14. If square-edge panels are used, edges must be supported between I-Joists with 2x6 blocking. Glue panels to blocking to minimize squeaks. Blocking is not required under structural finish flooring, such as wood strip flooring, or if a separate underlayment layer is installed.
15. Nail spacing: Space nails installed in the flange's top face in accordance with the applicable building code requirements or approved building plans.

FIGURE 1
TYPICAL NORDIC I-JOIST FLOOR FRAMING AND CONSTRUCTION DETAILS



All nails shown in the above details are assumed to be common wire nails unless otherwise noted. 3" (0.122" dia.) common spiral nails may be substituted for 2-1/2" (0.128" dia.) common wire nails. Framing lumber assumed to be Spruce-Pine-Fir No. 2 or better. Individual components not shown to scale for clarity.

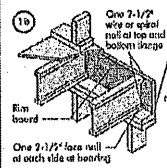
1a



Blocking Panel or Rim Joist	Maximum Factored Uniform Vertical Load* (plf)
NI Joists	3,300

*The uniform vertical load is limited to a joist depth of 16 inches or less and is based on standard term load duration. It shall not be used in the design of a bending member, such as joist, header, or rafter. For concentrated vertical load transfer, see detail 1d.

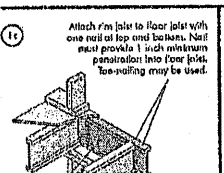
1b



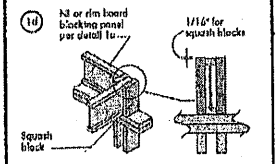
Blocking Panel or Rim Joist	Minimum Factored Uniform Vertical Load* (plf)
1-1/8" Rim Board Plus	8,090

*The uniform vertical load is limited to a rim board depth of 16 inches or less and is based on standard term load duration. It shall not be used in the design of a bearing member, such as joist, header, or rafter. For concentrated vertical load transfer, see detail 1d.

1c



1d



Pair of Squash Blocks	Maximum Factored Vertical per Pair of Squash Blocks (lbs)	
	3-1/2" wide	5-1/2" wide
2x Lumber	5,600	8,600
1-1/8" Rim Board Plus	4,300	6,600

Provide lateral bracing per detail 1a, 1b, or 1c

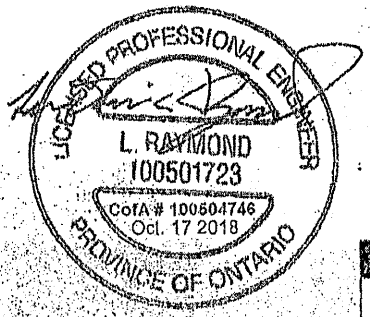
The construction details for residential designs are prone to changes.

Details released after April 2014 supersedes N-C301

Installation must comply with latest documentation on I-Joist and other Nordic products from the http://nordic.ca/

This document does not constitute a record of the structural integrity of the building nor suitability of the design assumptions made. Nordic Structures is responsible only for the structural adequacy of its component based on the design criteria and loadings shown on the calculation sheets.

Document prepared for the use of Stephanie Gon from Alpa, Ontario. (Nordic Request 1810-095)



MAXIMUM FLOOR SPANS

1. Maximum clear spans applicable to simple-span or multiple-span residential floor construction with a design live load of 40 psf and dead load of 15 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration and a live load deflection limit of L/480. For multiple-span applications, the end spans shall be 40% or more of the adjacent span.
2. Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 5/8 inch for a joist spacing of 19.2 inches or less, or 3/4 inch for joist spacing of 24 inches. Adhesive shall meet the requirements given in CSA8-71.26 Standard. No concrete topping or bridging element was assumed. Increased spans may be achieved with the used of gypsum and/or a row of blocking at mid-span.
3. Minimum bearing length shall be 1-3/4 inches for the end bearings, and 3-1/2 inches for the intermediate bearings.
4. Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.
5. This span chart is based on uniform loads. For applications with other than uniform loads, an engineering analysis may be required based on the use of the design properties.
6. Tables are based on Limit States Design per CAN/CSA O86-09 Standard, and NBC 2010.
7. SI units conversions: 1 inch = 25.4 mm; 1 foot = 0.305 m

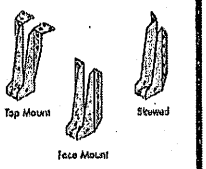
MAXIMUM FLOOR SPANS FOR NORDIC I-JOISTS SIMPLE AND MULTIPLE SPANS

9-1/2"	NI-40x	13'-1"	14'-2"	13'-9"	13'-5"	16'-3"	15'-1"	14'-10"	14'-5"
	NI-50	16'-1"	15'-2"	14'-8"	14'-3"	17'-6"	16'-3"	15'-10"	15'-6"
	NI-60	19'-5"	18'-4"	17'-9"	17'-3"	19'-7"	18'-5"	18'-0"	17'-6"
	NI-70	19'-5"	19'-1"	18'-10"	18'-7"	19'-7"	19'-4"	18'-7"	17'-8"
	NI-80	19'-5"	19'-1"	18'-10"	18'-7"	19'-7"	19'-4"	19'-1"	17'-8"
11-7/8"	NI-40x	16'-11"	16'-0"	15'-5"	15'-0"	18'-6"	17'-5"	16'-8"	16'-2"
	NI-50	19'-1"	17'-8"	16'-9"	16'-3"	20'-0"	18'-6"	17'-9"	17'-3"
	NI-60	19'-4"	17'-3"	16'-7"	16'-0"	20'-3"	18'-9"	18'-0"	18'-9"
	NI-70	19'-6"	19'-0"	17'-4"	17'-6"	21'-6"	19'-11"	19'-0"	19'-8"
	NI-80	19'-6"	19'-3"	17'-8"	17'-7"	21'-9"	20'-2"	19'-3"	19'-11"
14"	NI-50	20'-2"	18'-7"	17'-10"	17'-11"	22'-5"	20'-7"	19'-8"	19'-4"
	NI-60	20'-4"	18'-9"	17'-11"	17'-0"	22'-5"	20'-9"	19'-10"	20'-5"
	NI-70	20'-4"	20'-3"	19'-1"	18'-9"	22'-5"	20'-9"	19'-10"	20'-5"
	NI-80	20'-4"	20'-3"	19'-5"	18'-9"	22'-5"	20'-9"	19'-10"	20'-5"
16"	NI-50	20'-10"	19'-2"	18'-5"	18'-5"	22'-7"	20'-11"	20'-0"	20'-10"
	NI-60	21'-7"	20'-0"	19'-1"	19'-2"	23'-10"	22'-1"	21'-1"	21'-10"
	NI-70	21'-11"	20'-3"	19'-4"	19'-5"	24'-3"	22'-5"	21'-4"	22'-3"
	NI-80	21'-11"	20'-3"	19'-4"	19'-5"	24'-3"	22'-5"	21'-4"	22'-3"
19.2"	NI-60	22'-5"	20'-8"	19'-9"	19'-7"	24'-9"	22'-10"	21'-10"	21'-10"
	NI-70	22'-5"	20'-11"	19'-11"	19'-11"	24'-9"	23'-1"	22'-0"	22'-10"
	NI-80	22'-5"	20'-11"	19'-11"	20'-0"	24'-9"	23'-1"	22'-0"	22'-10"
24"	NI-60	22'-5"	20'-8"	19'-9"	19'-10"	24'-9"	22'-9"	21'-9"	22'-6"
	NI-70	23'-6"	21'-9"	20'-9"	20'-10"	26'-0"	24'-0"	22'-11"	23'-9"
	NI-80	23'-11"	22'-1"	21'-1"	21'-2"	26'-5"	24'-5"	23'-3"	24'-1"
	NI-90	24'-8"	22'-9"	21'-9"	21'-10"	26'-11"	24'-10"	23'-9"	23'-9"
	NI-90x	24'-8"	22'-9"	21'-9"	21'-10"	27'-3"	28'-2"	24'-0"	24'-10"

CCMC EVALUATION REPORT 13032-R

I-JOIST HANGERS

1. Hangers shown illustrate the three most commonly used metal hangers to support I-joists.
2. All nailing must meet the hanger manufacturer's recommendations.
3. Hangers should be selected based on the joist depth, flange width and load capacity based on the maximum spans.
4. Web stiffeners are required when the sides of the hangers do not laterally brace the top flange of the I-joist.



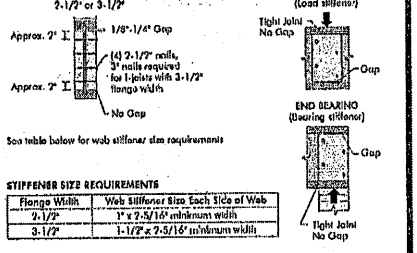
WEB STIFFENERS

RECOMMENDATIONS:

- A *bearing stiffener* is required in all engineered applications with factored reactions greater than shown in the I-joist properties table found of the *I-Joist Construction Guide* (C101) The gap between the stiffener and the flange is at the top.
- A *bearing stiffener* is required when the I-joist is supported in a hanger and the sides of the hanger do not extend up to, and support, the top flange. The gap between the stiffener and flange is at the top.
- A *load stiffener* is required at locations where a factored concentrated load greater than 2,370 lbs is applied to the top flange between supports, or in the case of a cantilever, anywhere between the cantilever tip and the support. These values are for standard term load duration, and may be adjusted for other load durations as permitted by the code. The gap between the stiffener and the flange is at the bottom.

SI units conversion: 1 inch = 25.4 mm

FIGURE 2
WEB STIFFENER INSTALLATION DETAILS

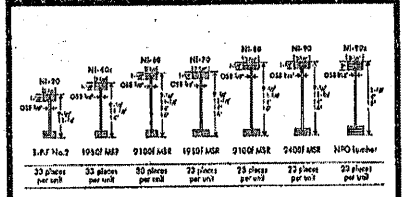


See table below for web stiffener size requirements

STIFFENER SIZE REQUIREMENTS

Flange Width	Web Stiffener Size Each Side of Web
2-1/2"	1" x 2-5/16" minimum width
3-1/2"	1-1/2" x 2-5/16" minimum width

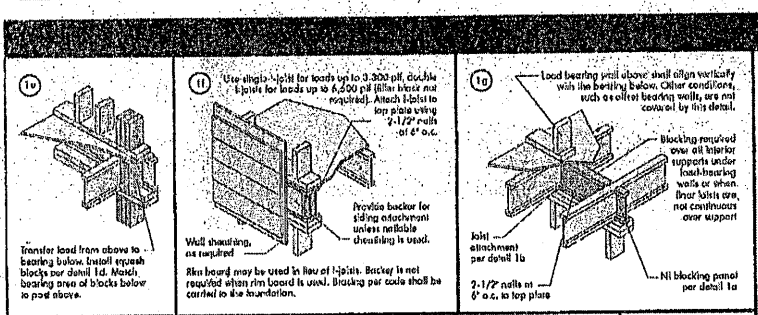
NORDIC I-JOIST SERIES



1.8E No.2	1950f MSR	2100f MSR	1950f MSR	2100f MSR	2400f MSR	NRO lumber
33 pieces per unit	33 pieces per unit	30 pieces per unit	22 pieces per unit	25 pieces per unit	25 pieces per unit	25 pieces per unit

Chantiers Chibougamau Ltd. harvests its own trees, which enables Nordic products to adhere to strict quality control procedures throughout the manufacturing process. Every photo of the operation, from forest to the finished product, reflects our commitment to quality.

Nordic Engineered Wood I-joists use only finger-jointed black spruce lumber in their flanges, ensuring consistent quality, superior strength, and longer span carrying capacity.

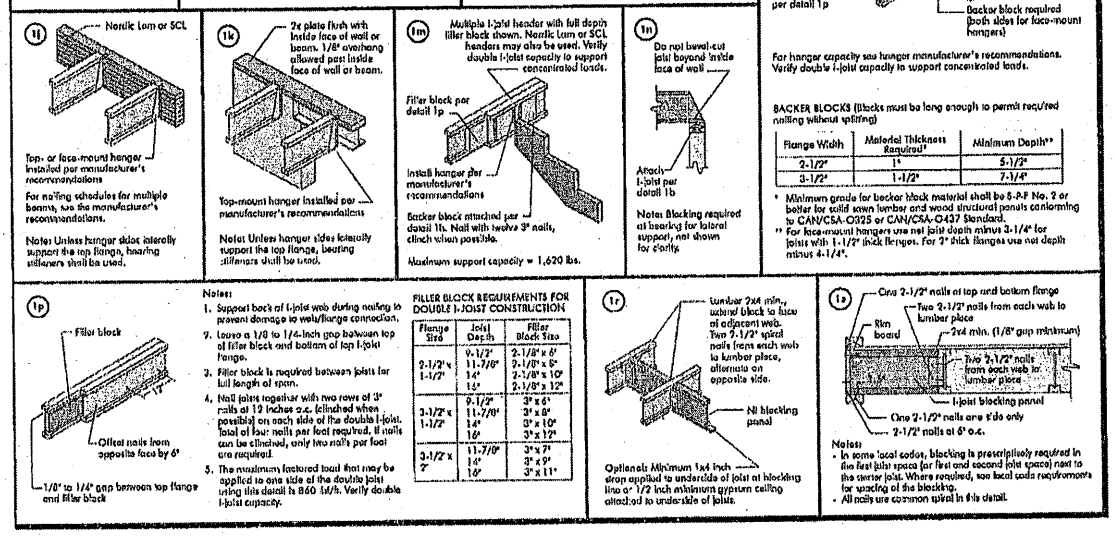


BACKER BLOCKS (Blocks must be long enough to permit required nailing without splitting)

Flange Width	Material Thickness Required*	Minimum Depth**
2-1/2"	1"	5-1/2"
3-1/2"	1-1/2"	7-1/4"

* Minimum grade for backer block material shall be S-P-F No. 2 or better for solid sawn lumber and wood structural panels conforming to CAN/CSA-O325 or CAN/CSA-O437 Standard.

** For face-mount hangers use net joist depth minus 3-1/4" for joists with 1-1/2" thick flanges. For 2" thick flanges use net depth minus 4-1/4".



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Document prepared for the use of Stephanie Gon from Alpa, Ontario. (Nordic Request 1810-095)

LICENSED PROFESSIONAL ENGINEER
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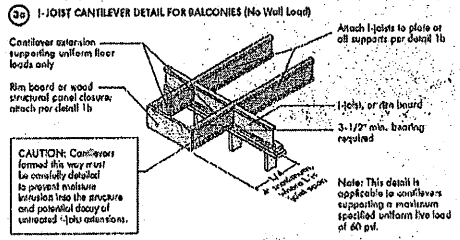
CofA # 100504746
Oct. 17 2018
PROVINCE OF ONTARIO

CANTILEVER DETAILS FOR BALCONIES (NO WALL LOAD)

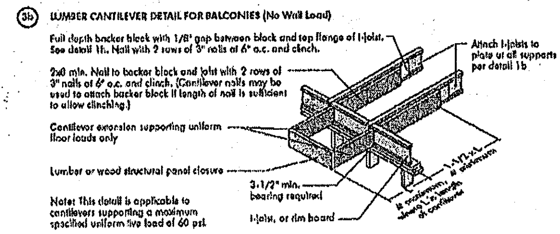
(3a) I-JOIST CANTILEVER DETAIL FOR BALCONIES (No Wall Load)



CAUTION: Cantilevers formed this way must be carefully detailed to prevent moisture intrusion into the structure and potential decay of untreated I-joist extensions.

Note: This detail is applicable to cantilevers supporting a maximum specified uniform live load of 60 psf.

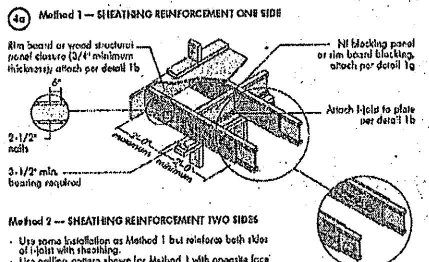
(3b) LUMBER CANTILEVER DETAIL FOR BALCONIES (No Wall Load)



Note: This detail is applicable to cantilevers supporting a maximum specified uniform live load of 60 psf.

CANTILEVER DETAILS FOR VERTICAL BUILDING OFFSET (CONCENTRATED WALL LOAD)

(4a) Method 1 — SHEATHING REINFORCEMENT ONE SIDE

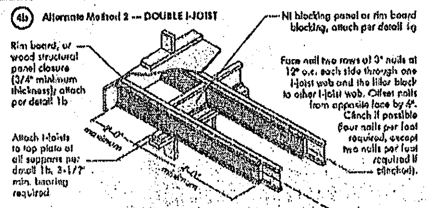


Method 2 — SHEATHING REINFORCEMENT TWO SIDES

- Use same installation as Method 1 but reinforce both sides of I-joist with sheathing.
- Use nailing pattern shown for Method 1 with opposite face nailing offset by 3".

Note: Canadian softwood plywood sheathing or equivalent (minimum thickness 3/4") required on sides of joist. Depth shall match the full height of the joist. Nail with 2-1/2" nails at 6" o.c., top and bottom flange. Install with face grain horizontal. Attach I-joist to plate at all supports per detail 1b. Verify reinforced I-joist capacity.

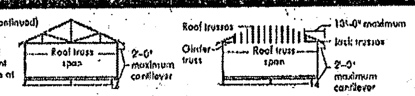
(4b) Alternate Method 2 — DOUBLE I-JOIST



Block I-joists together with filler blocks for the full length of the reinforcement. For I-joist flange widths greater than 3 inches place an additional row of 3" nails along the centerline of the reinforcing panel from each side. Clinch when possible.

FIGURE 4 (continued)

See table below for NI reinforcement requirements at cantilever.



For hip roofs with the jack trusses running parallel to the cantilevered floor joists, the I-joist reinforcement requirements for a span of 26 ft. shall be permitted to be used.

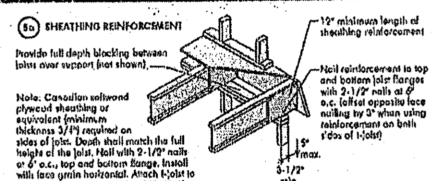
CANTILEVER REINFORCEMENT METHODS ALLOWED

9-1/2	26	N	N	1	2	N	1	2	X	N	2	X	X
	28	N	N	1	X	N	1	2	X	N	2	X	X
	30	N	1	1	X	N	1	2	X	1	2	X	X
	32	N	1	2	X	N	2	X	X	1	X	X	X
	34	N	1	2	X	N	2	X	X	1	X	X	X
	36	N	1	2	X	1	2	X	X	1	X	X	X
11-7/8	26	1	N	N	1	1	N	1	2	2	N	1	2
	28	1	N	N	1	2	N	1	2	2	1	1	X
	30	1	N	N	1	2	N	1	2	X	1	2	X
	32	1	N	1	1	2	N	1	2	X	1	2	X
	34	1	N	1	2	2	1	1	X	X	1	2	X
	36	2	N	1	2	X	1	2	X	X	1	2	X
	38	2	N	1	2	X	1	2	X	X	2	X	X
14	26	N	N	N	N	N	N	N	1	N	N	N	1
	28	N	N	N	N	N	N	N	1	N	N	1	1
	30	N	N	N	N	N	N	N	1	N	N	1	2
	32	N	N	N	1	N	N	N	1	N	N	1	2
	34	N	N	N	1	N	N	1	1	N	N	1	2
	36	N	N	N	1	N	N	1	2	N	1	1	2
	38	N	N	N	1	N	N	1	2	N	1	1	X
	40	N	N	N	1	N	N	1	2	N	1	2	X
16	26	N	N	N	N	N	N	N	N	N	N	N	1
	28	N	N	N	N	N	N	N	1	N	N	N	1
	30	N	N	N	N	N	N	N	1	N	N	N	1
	32	N	N	N	N	N	N	N	1	N	N	1	1
	34	N	N	N	N	N	N	N	1	N	N	1	2
	36	N	N	N	1	N	N	N	1	N	N	1	2
	38	N	N	N	1	N	N	N	1	N	N	1	2
	40	N	N	N	1	N	N	1	2	N	N	1	2

1. N = No reinforcement required.
   1 = NI reinforced with 3/4" wood structural panel on one side only.
   2 = NI reinforced with 3/4" wood structural panel on both sides, or double I-joist.
   X = Try a deeper joist or closer spacing.
2. Maximum design load shall be: 15 psf roof dead load, 55 psf floor total load, and 80 plf wall load. Wall load is based on 3'-0" maximum width window or door openings. For larger openings, or multiple 3'-0" width openings spaced less than 6'-0" o.c., additional joists beneath the opening's cripple studs may be required.
3. Table applies to joists 12" to 24" o.c. that meet the floor span requirements for a design live load of 40 psf and dead load of 15 psf, and a live load deflection limit of L/480. Use 12" o.c. requirements for lesser spacing.
4. For conventional roof construction using a ridge beam, the Roof Truss Span column above is equivalent to the distance between the supporting wall and the ridge beam. When the roof is framed using a ridge board, the Roof Truss Span is equivalent to the distance between the supporting walls as if a truss is used.
5. Cantilevered joists supporting girder trusses or roof beams may require additional reinforcing.

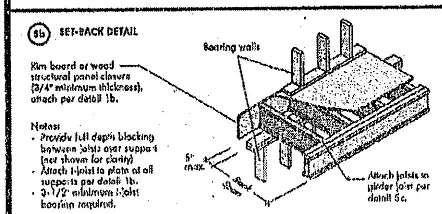
BRICK CANTILEVER DETAILS FOR VERTICAL BUILDING OFFSET (CONCENTRATED WALL LOAD)

(5a) SHEATHING REINFORCEMENT



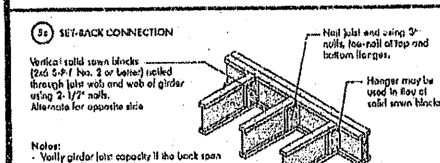
Note: Canadian softwood plywood sheathing or equivalent (minimum thickness 3/4") required on sides of joist. Depth shall match the full height of the joist. Nail with 2-1/2" nails at 6" o.c., top and bottom flange. Install with face grain horizontal. Attach I-joist to plate at all supports per detail 1b. Verify reinforced I-joist capacity.

(5b) SET-BACK DETAIL



Notes:
- Provide full depth blocking between joists over support (not shown for clarity).
- Attach I-joist to plate at all supports per detail 1b.
- 3-1/2" minimum I-joist bearing required.

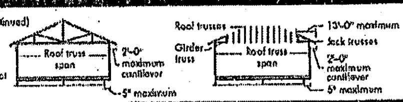
(5c) SET-BACK CONNECTION



Notes:
- Verify girder joist capacity if the back span exceeds the joist spacing.
- Attach double I-joist per detail 1p, if required.

FIGURE 5 (continued)

See table below for NI reinforcement requirements at cantilever.



For hip roofs with the jack trusses running parallel to the cantilevered floor joists, the I-joist reinforcement requirements for a span of 26 ft. shall be permitted to be used.

BRICK CANTILEVER REINFORCEMENT METHODS ALLOWED

9-1/2	26	1	X	X	X	2	X	X	X	2	X	X	X
	28	1	X	X	X	2	X	X	X	X	X	X	X
	30	1	X	X	X	2	X	X	X	X	X	X	X
	32	2	X	X	X	2	X	X	X	X	X	X	X
	34	2	X	X	X	X	X	X	X	X	X	X	X
	36	2	X	X	X	X	X	X	X	X	X	X	X
11-7/8	26	X	2	X	X	X	X	X	X	X	X	X	X
	28	X	2	X	X	X	X	X	X	X	X	X	X
	30	X	2	X	X	X	X	X	X	X	X	X	X
	32	X	2	X	X	X	X	X	X	X	X	X	X
	34	X	X	X	X	X	X	X	X	X	X	X	X
	36	X	X	X	X	X	X	X	X	X	X	X	X
	38	X	X	X	X	X	X	X	X	X	X	X	X
14	26	N	2	X	X	1	X	X	X	1	X	X	X
	28	N	2	X	X	1	X	X	X	2	X	X	X
	30	1	2	X	X	1	X	X	X	2	X	X	X
	32	1	X	X	X	1	X	X	X	2	X	X	X
	34	1	X	X	X	2	X	X	X	2	X	X	X
	36	1	X	X	X	2	X	X	X	X	X	X	X
	38	1	X	X	X	2	X	X	X	X	X	X	X
	40	1	X	X	X	2	X	X	X	X	X	X	X
16	26	1	2	X	X	1	X	X	X	1	X	X	X
	28	1	2	X	X	1	X	X	X	2	X	X	X
	30	1	X	X	X	1	X	X	X	2	X	X	X
	32	1	X	X	X	2	X	X	X	2	X	X	X
	34	1	X	X	X	2	X	X	X	X	X	X	X
	36	1	X	X	X	2	X	X	X	X	X	X	X
	38	1	X	X	X	2	X	X	X	X	X	X	X
	40	2	X	X	X	2	X	X	X	X	X	X	X

1. N = No reinforcement required.
   1 = NI reinforced with 3/4" wood structural panel on one side only.
   2 = NI reinforced with 3/4" wood structural panel on both sides, or double I-joist.
   X = Try a deeper joist or closer spacing.
2. Maximum design load shall be: 15 psf roof dead load, 55 psf floor total load, and 80 plf wall load. Wall load is based on 3'-0" maximum width window or door openings. For larger openings, or multiple 3'-0" width openings spaced less than 6'-0" o.c., additional joists beneath the opening's cripple studs may be required.
3. Table applies to joists 12" to 24" o.c. that meet the floor span requirements for a design live load of 40 psf and dead load of 15 psf, and a live load deflection limit of L/480. Use 12" o.c. requirements for lesser spacing.
4. For conventional roof construction using a ridge beam, the Roof Truss Span column above is equivalent to the distance between the supporting wall and the ridge beam. When the roof is framed using a ridge board, the Roof Truss Span is equivalent to the distance between the supporting walls as if a truss is used.
5. Cantilevered joists supporting girder trusses or roof beams may require additional reinforcing.

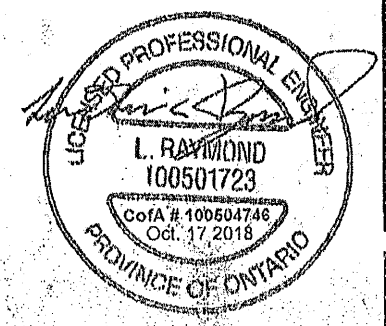
The construction details for residential designs are prone to changes.

Details released after April 2014 supersedes N-C301

Installation must comply with latest documentation on I-Joist and other Nordic products from the http://nordic.ca/

This document does not constitute a record of the structural integrity of the building nor suitability of the design assumptions made. Nordic Structures is responsible only for the structural adequacy of its component based on the design criteria and loadings shown on the calculation sheets.

Document prepared for the use of Stephanie Gon from Alpa, Ontario. (Nordic Request 1810-095)

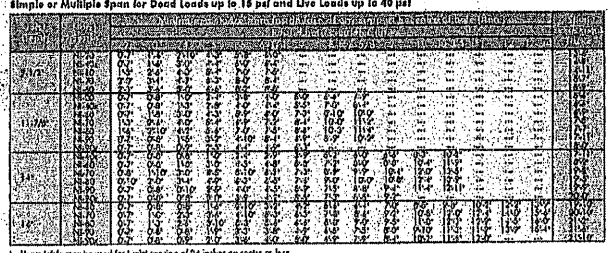


WEB HOLES

RULES FOR CUTTING HOLES AND DUCT CHASE OPENINGS:

1. The distance between the inside edge of the support and the centreline of any hole or duct chase opening shall be in compliance with the requirements of Table 1 or 2, respectively.
2. I-joist top and bottom flanges must NEVER be cut, notched, or otherwise modified.
3. Whenever possible, field-cut holes should be centred on the middle of the web.
4. The maximum size hole or the maximum depth of a duct chase opening that can be cut into an I-joist web shall equal the clear distance between the flanges of the I-joist minus 1/4 inch. A minimum of 1/8 inch should always be maintained between the top or bottom of the hole or opening and the adjacent I-joist flange.
5. The sides of square holes or largest sides of rectangular holes should not exceed 3/4 of the diameter of the maximum round hole permitted at that location.
6. Where more than one hole is necessary, the distance between adjacent hole edges shall exceed twice the diameter of the largest round hole or twice the size of the largest square hole for twice the length of the largest side of the largest rectangular hole or duct chase opening) and each hole and duct chase opening shall be sized and located in compliance with the requirements of Tables 1 and 2, respectively.
7. A knockout is not considered a hole, may be utilized anywhere it occurs, and may be ignored for purposes of calculating minimum distances between holes and/or duct chase openings.
8. Holes measuring 1-1/2 inches or smaller shall be permitted anywhere in a cantilevered section of a joist. Holes of greater size may be permitted subject to verification.
9. A 1-1/2 inch hole or smaller can be placed anywhere in the web provided that it meets the requirements of rule number 6 above.
10. All holes and duct chase openings shall be cut in a workmen-like manner in accordance with the restrictions listed above and as illustrated in Figure 7.
11. Limit three maximum size holes per span, of which one may be a duct chase opening.
12. A group of round holes at approximately the same location shall be permitted if they meet the requirements for a single round hole circumscribed around them.

TABLE 1
LOCATION OF CIRCULAR HOLES IN JOIST WEBS
Simple or Multiple Span for Dead Loads up to 15 psf and Live Loads up to 40 psf



1. Above table may be used for I-joist spacing of 24 inches on centre or less.
2. Hole location distance is measured from inside face of supports to centre of hole.
3. Distances in this chart are based on uniformly loaded joists.

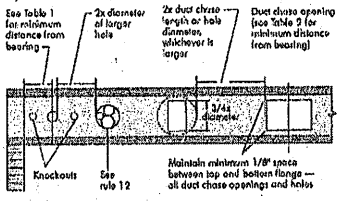
OPTIONAL:

The above table is based on the I-joists used at their maximum spans. If the I-joists are placed at less than their full maximum span (see Maximum Floor Spans), the minimum distance from the centreline of the hole to the face of any support (D) as given above may be reduced as follows:

$$D_{reduced} = \frac{L_{actual}}{SAF} \times D$$

Where: $D_{reduced}$ = Distance from the inside face of any support to centre of hole, reduced by less-than-maximum span applications (ft). The reduced distance shall not be less than 5 inches from the face of the support to edge of the hole.
L_{actual} = The actual measured span distance between the inside faces of supports (ft).
SAF = Span Adjustment Factor given in the table.
D = The minimum distance from the inside face of any support to centre of hole from the table.
If L_{actual}/SAF is greater than 1, use 1 in the above calculation for L_{actual}/SAF.

FIGURE 7
FIELD-CUT HOLE LOCATOR



A knockout is NOT considered a hole, may be utilized wherever it occurs and may be ignored for purposes of calculating minimum distances between holes.

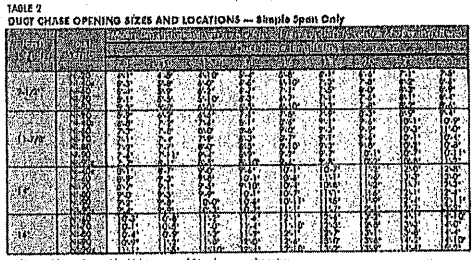
Knockouts are prescored holes provided for the contractor's convenience to install electrical or small plumbing lines. They are 1-1/2 inches in diameter, and are spaced 15 inches on centre along the length of the I-joist. Where possible, it is preferable to use knockouts instead of field-cut holes.

Never drill, cut or notch the flange, or over-cut the web.

Holes in webs should be cut with a sharp saw.

For rectangular holes, avoid overcutting the corners, as this can cause unnecessary stress concentrations. Slightly rounding the corners is recommended. Starting the rectangular hole by drilling a 1-inch diameter hole in each of the four corners and then making the cuts between the holes is another good method to minimize damage to the I-joist.

TABLE 2
DUCT CHASE OPENING SIZES AND LOCATIONS — Simple Span Only



1. Above table may be used for I-joist spacing of 24 inches on centre or less.
2. Duct chase opening location distance is measured from inside face of supports to centre of opening.
3. The above table is based on simple-span joists only. For other applications, contact your local distributor.
4. Distances are based on uniformly loaded floor joists that meet the span requirements for a design live load of 40 psf and dead load of 15 psf, and a live load deflection limit of L/480. For other applications, contact your local distributor.

INSTALLING THE GLUED FLOOR SYSTEM

1. Wipe any mud, dirt, water, or ice from I-joint flanges before gluing.
2. Snap a chalk line across the I-joists four feet in from the wall for panel edge alignment and as a boundary for spreading glue.
3. Spread only enough glue to lay one or two panels at a time, or follow specific recommendations from the glue manufacturer.
4. Lay the first panel with tongue side to the wall, and nail in place. This protects the tongue of the next panel from damage when tapped into place with a block and sledgehammer.
5. Apply a continuous line of glue (about 1/4-inch diameter) to the top flange of a single I-joist. Apply glue in a winding pattern on wide areas, such as with double I-joists.
6. Apply two lines of glue on I-joists where panel ends butt to assure proper gluing of each end.
7. After the first row of panels is in place, spread glue in the groove of one or two panels at a time before laying the next row. Glue line may be continuous or spaced, but avoid squeeze-out by applying a thinner line (1/8 inch) than used on I-joist flanges.
8. Tap the second row of panels into place, using a block to protect groove edges.
9. Stagger end joints in each succeeding row of panels. A 1/8-inch space between all end joints and 1/8-inch at all edges, including T&G edges, is recommended. (Use a spacer tool or an 2-1/2" common nail to assure accurate and consistent spacing.)
10. **Complete all nailing of each panel before glue sets. Check the manufacturer's recommendations for cure time. (Warm weather accelerates glue setting.) Use 2" ring- or screw-shank nails for panels 3/4-inch thick or less, and 2-1/2" ring- or screw-shank nails for thicker panels. Space nails per the table below. Closer nail spacing may be required by some codes, or for diaphragm construction. The finished deck can be walked on right away and will carry construction loads without damage to the glue bond.**

FASTENERS FOR SHEATHING AND SUBFLOORING[1]

16	5/8	2"	1-3/4"	2"	6"	12"
20	5/8	2"	1-3/4"	2"	6"	12"
24	3/4	2"	1-3/4"	2"	6"	12"

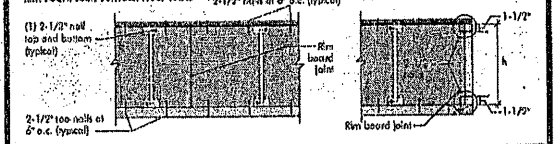
1. Fasteners of sheathing and subflooring shall conform to the above table.
2. Staples shall not be less than 1/16-inch in diameter or thickness, with not less than a 7/8-inch crown driven with the crown parallel to framing.
3. Flooring screws shall not be less than 1/8-inch in diameter.
4. Special conditions may impose heavy traffic and concentrated loads that require construction in excess of the minimums shown.
5. Use only adhesives conforming to CAN/CSA-O 1.26 Standard, Adhesives for Field-Gluing Plywood to Lumber Framing for Floor System, applied in accordance with the manufacturer's recommendations. If OSB panels with sealed surfaces and edges are to be used, use only solvent-based glues; check with panel manufacturer.

Ref.: NRC-CNRC, *National Building Code of Canada 2010*, Table 9.23.3.5.

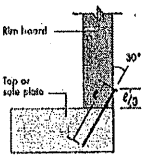
IMPORTANT NOTE:
Floor sheathing must be field glued to the I-joist flanges in order to achieve the maximum spans shown in this document. If sheathing is nailed only, I-joist spans must be verified with your local distributor.

RIM BOARD INSTALLATION DETAILS

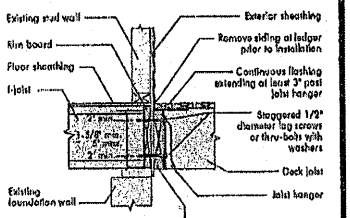
9a ATTACHMENT DETAILS WHERE RIM BOARDS ABUT

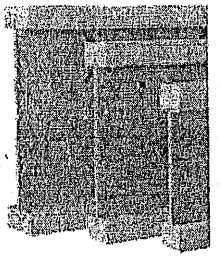


9b TOE-NAIL CONNECTION AT RIM BOARD



9c 2X LEDGER TO RIM BOARD ATTACHMENT DETAIL



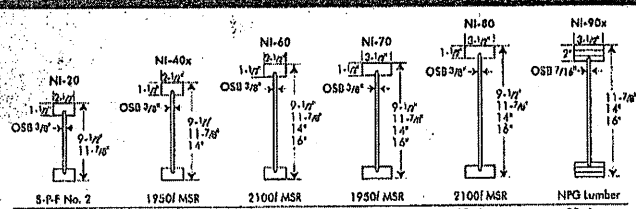


CONSTRUCTION DETAILS FOR RESIDENTIAL FLOORS

NORDIC ENGINEERED WOOD

www.nordicewp.com

FSC www.fsc.org



Refer to the *Installation Guide for Residential Floors* for additional information.
CCMC EVALUATION REPORT 13032-R

WEB HOLE SPECIFICATIONS

RULES FOR CUTTING HOLES AND DUCT CHASE OPENINGS:

1. The distance between the inside edge of the support and the centreline of any hole or duct chase opening shall be in compliance with the requirements of Table 1 or 2, respectively.
2. I-joist top and bottom flanges must NEVER be cut, notched, or otherwise modified.
3. Whenever possible, field-cut holes should be centred on the middle of the web.
4. The maximum size hole or the maximum depth of a duct chase opening that can be cut into an I-joist web shall equal the clear distance between the flanges of the I-joist minus 1/4 inch. A minimum of 1/8 inch should always be maintained between the top or bottom of the hole or opening and the adjacent I-joist flange.
5. The sides of square holes or longest sides of rectangular holes should not exceed 3/4 of the diameter of the maximum round hole permitted at that location.
6. Where more than one hole is necessary, the distance between adjacent hole edges shall exceed twice the diameter of the largest round hole or twice the size of the largest square hole *(or twice the length of the longest side of the longest rectangular hole or duct chase opening)* and each hole and duct chase opening shall be sized and located in compliance with the requirements of Tables 1 and 2, respectively.
7. A knockout is not considered a hole, may be utilized anywhere it occurs, and may be ignored for purposes of calculating minimum distances between holes and/or duct chase openings.
8. Holes measuring 1-1/2 inches or smaller are permitted anywhere in a cantilevered section of a joist. Holes of greater size may be permitted subject to verification.
9. A 1-1/2 inch hole or smaller can be placed anywhere in the web provided that it meets the requirements of rule number 6 above.
10. All holes and duct chase openings shall be cut in a workmon-like manner in accordance with the restrictions listed above and as illustrated in Figure 7.
11. Limit three maximum size holes per span, of which one may be a duct chase opening.
12. A group of round holes at approximately the same location shall be permitted if they meet the requirements for a single round hole circumscribed around them.

TABLE 1

LOCATION OF CIRCULAR HOLES IN JOIST WEBS

Simple or Multiple Span for Dead Loads up to 15 psf and Live Loads up to 40 psf

Joist Depth	Joist Series	Minimum Distance from Inside Face of Any Support to Centre of Hole (ft - in.) — Round Hole Diameter (in.)													
		2	3	4	5	6	6-1/4	7	8	8-5/8	9	10	10-3/4	11	12	12-3/4
9-1/2"	NI-20	0'-7"	1'-6"	2'-10"	4'-3"	5'-8"	6'-0"	---	---	---	---	---	---	---	---	---
	NI-40x	0'-7"	1'-6"	3'-0"	4'-4"	6'-0"	6'-4"	---	---	---	---	---	---	---	---	---
	NI-60	1'-3"	2'-6"	4'-0"	5'-4"	7'-0"	7'-5"	---	---	---	---	---	---	---	---	---
	NI-70	2'-0"	3'-4"	4'-9"	6'-3"	8'-0"	8'-4"	---	---	---	---	---	---	---	---	---
	NI-80	2'-3"	3'-6"	5'-0"	6'-5"	8'-2"	8'-8"	---	---	---	---	---	---	---	---	---
11-7/8"	NI-20	0'-7"	0'-8"	1'-0"	2'-4"	3'-8"	4'-0"	5'-0"	6'-6"	7'-9"	---	---	---	---	---	---
	NI-40x	0'-7"	0'-8"	1'-3"	2'-8"	4'-0"	4'-4"	5'-5"	7'-0"	8'-4"	---	---	---	---	---	---
	NI-60	0'-7"	1'-8"	3'-0"	4'-3"	5'-9"	6'-0"	7'-3"	8'-10"	10'-0"	---	---	---	---	---	---
	NI-70	1'-3"	2'-6"	4'-0"	5'-4"	6'-9"	7'-2"	8'-4"	10'-0"	11'-2"	---	---	---	---	---	---
	NI-80	1'-6"	2'-10"	4'-2"	5'-6"	7'-0"	7'-5"	8'-6"	10'-3"	11'-4"	---	---	---	---	---	---
	NI-90x	0'-7"	0'-0"	0'-9"	2'-5"	4'-4"	4'-9"	6'-3"	---	---	---	---	---	---	---	---
14"	NI-40x	0'-7"	0'-0"	0'-8"	1'-0"	2'-4"	2'-9"	3'-9"	5'-2"	6'-0"	6'-6"	8'-3"	10'-2"	---	---	---
	NI-60	0'-7"	0'-0"	1'-8"	3'-0"	4'-3"	4'-8"	5'-8"	7'-2"	8'-0"	8'-8"	10'-4"	11'-9"	---	---	---
	NI-70	0'-8"	1'-10"	3'-0"	4'-5"	5'-10"	6'-2"	7'-3"	8'-9"	9'-9"	10'-4"	12'-0"	13'-5"	---	---	---
	NI-80	0'-10"	2'-0"	3'-4"	4'-9"	6'-2"	6'-5"	7'-6"	9'-0"	10'-0"	10'-8"	12'-4"	13'-9"	---	---	---
	NI-90x	0'-7"	0'-0"	0'-8"	2'-0"	3'-9"	4'-2"	5'-5"	7'-3"	8'-5"	9'-2"	---	---	---	---	---
16"	NI-60	0'-7"	0'-8"	0'-8"	1'-6"	2'-10"	3'-2"	4'-2"	5'-6"	6'-4"	7'-0"	8'-5"	9'-8"	10'-2"	12'-2"	13'-9"
	NI-70	0'-7"	1'-0"	2'-0"	3'-4"	4'-10"	5'-3"	6'-3"	7'-8"	8'-6"	9'-2"	10'-8"	12'-0"	12'-4"	14'-0"	15'-6"
	NI-80	0'-7"	1'-2"	2'-6"	3'-10"	5'-3"	5'-6"	6'-6"	8'-0"	9'-0"	9'-5"	11'-0"	12'-3"	12'-9"	14'-5"	16'-0"
	NI-90x	0'-7"	0'-8"	0'-9"	2'-0"	3'-6"	4'-0"	5'-0"	6'-9"	7'-9"	8'-4"	10'-2"	11'-6"	12'-0"	---	---

1. Above table may be used for I-joist spacing of 24 inches on centre or less.
2. Hole location distance is measured from inside face of supports to centre of hole.
3. Distances in this chart are based on uniformly loaded joists.
4. The above table is based on the I-joists being used at their maximum spans. The minimum distance as given above may be reduced for shorter spans; contact your local distributor.

TABLE 2

DUCT CHASE OPENING SIZES AND LOCATIONS

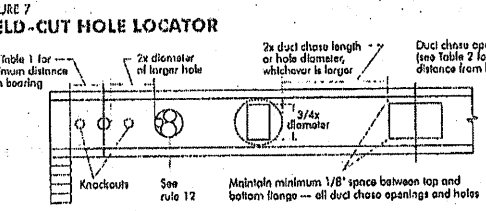
Simple Span Only

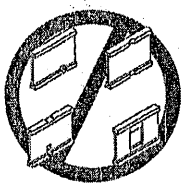
Joist Depth	Joist Series	Minimum Distance from Inside Face of Supports to Centre of Opening (ft - in.) — Duct Chase Length (in.)								
		8	10	12	14	16	18	20	22	24
9-1/2"	NI-20	4'-1"	4'-5"	4'-10"	5'-4"	5'-8"	6'-1"	6'-6"	7'-1"	7'-5"
	NI-40x	5'-2"	5'-8"	6'-0"	6'-5"	6'-10"	7'-3"	7'-8"	8'-2"	8'-6"
	NI-60	5'-4"	5'-9"	6'-2"	6'-7"	7'-1"	7'-5"	8'-0"	8'-3"	8'-9"
	NI-70	5'-1"	5'-5"	5'-10"	6'-3"	6'-7"	7'-1"	7'-6"	8'-1"	8'-4"
	NI-80	5'-3"	5'-8"	6'-0"	6'-5"	6'-10"	7'-3"	7'-8"	8'-2"	8'-6"
11-7/8"	NI-20	5'-9"	6'-2"	6'-6"	7'-1"	7'-5"	7'-9"	8'-3"	8'-9"	9'-4"
	NI-40x	6'-8"	7'-2"	7'-6"	8'-1"	8'-6"	9'-1"	9'-6"	10'-1"	10'-9"
	NI-60	7'-3"	7'-8"	8'-0"	8'-6"	9'-0"	9'-3"	9'-9"	10'-3"	11'-0"
	NI-70	7'-1"	7'-4"	7'-9"	8'-3"	8'-7"	9'-1"	9'-6"	10'-1"	10'-4"
	NI-80	7'-2"	7'-7"	8'-0"	8'-5"	8'-10"	9'-3"	9'-8"	10'-2"	10'-8"
	NI-90x	7'-7"	8'-1"	8'-5"	8'-10"	9'-4"	9'-8"	10'-2"	10'-8"	11'-2"
14"	NI-40x	8'-1"	8'-7"	9'-0"	9'-6"	10'-1"	10'-7"	11'-2"	12'-0"	12'-0"
	NI-60	8'-9"	9'-3"	9'-8"	10'-1"	10'-6"	11'-1"	11'-6"	13'-3"	13'-0"
	NI-70	8'-7"	9'-1"	9'-5"	9'-10"	10'-4"	10'-8"	11'-2"	11'-7"	12'-3"
	NI-80	9'-0"	9'-3"	9'-9"	10'-1"	10'-7"	11'-1"	11'-6"	12'-1"	12'-6"
	NI-90x	9'-4"	9'-9"	10'-3"	10'-7"	11'-1"	11'-7"	12'-1"	12'-7"	13'-2"
16"	NI-60	10'-3"	10'-8"	11'-2"	11'-6"	12'-1"	12'-6"	13'-2"	14'-1"	14'-10"
	NI-70	10'-1"	10'-5"	11'-0"	11'-4"	11'-10"	12'-3"	12'-8"	13'-3"	14'-0"
	NI-80	10'-4"	10'-9"	11'-3"	11'-9"	12'-1"	12'-7"	13'-1"	13'-8"	14'-4"
	NI-90x	11'-1"	11'-5"	11'-10"	12'-4"	12'-10"	13'-2"	13'-9"	14'-4"	15'-2"

1. Above table may be used for I-joist spacing of 24 inches on centre or less.
2. Duct chase opening location distance is measured from inside face of supports to centre of opening.
3. The above table is based on simple-span joists only. For other applications, contact your local distributor.
4. Distances are based on uniformly loaded floor joists that meet the span requirements for a design live load of 40 psf and dead load of 15 psf, and a live load deflection limit of L/480.
5. The above table is based on the I-joists being used at their maximum spans. The minimum distance as given above may be reduced for shorter spans; contact your local distributor.

FIGURE 7

FIELD-CUT HOLE LOCATOR





Knockouts are prescored holes provided for the contractor's convenience to install electrical or small plumbing lines. They are 1-1/2 inches in diameter, and are spaced 15 inches on centre along the length of the I-joist. Where possible, it is preferable to use knockouts instead of field-cut holes.

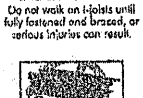
Never drill, cut or notch the flange, or over-cut the web.

Holes in webs should be cut with a sharp saw.

For rectangular holes, avoid over-cutting the corners, as this can cause unnecessary stress concentrations. Slightly rounding the corners is recommended. Starting the rectangular hole by drilling a 1-inch diameter hole in each of the four corners and then making the cuts between the holes is another good method to minimize damage to the I-joist.

SAFETY AND CONSTRUCTION PRECAUTIONS

Do not walk on I-joists until fully fastened and braced, or serious injuries can result.



Never stack building materials over unsheathed I-joists. Once sheathed, do not over-stress I-joists with concentrated loads from building materials.

WARNING: I-joists are not stable until completely installed, and will not carry any load until fully braced and sheathed.

AVOID ACCIDENTS BY FOLLOWING THESE IMPORTANT GUIDELINES:

1. Brace and nail each I-joist as it is installed, using hangers, blocking panels, rim board, and/or cross-bridging at joist ends. When I-joists are applied continuous over interior supports and a load-bearing wall is planned at that location, blocking will be required at the interior support.
2. When the building is completed, the floor sheathing will provide lateral support for the top flanges of the I-joists. Until this sheathing is applied, temporary bracing, often called strut, or temporary sheathing must be applied to prevent I-joist rollover or buckling.
   - Temporary bracing or struts must be 1x4 inch minimum, at least 8 feet long and spaced no more than 8 feet on centre, and must be secured with a minimum of two 2-1/2" nails fastened to the top surface of each I-joist. Nail the bracing to a lateral restraint at the end of each bay. Lap ends of adjoining bracing over at least two I-joists.
   - Or, sheathing (temporary or permanent) can be nailed to the top flange of the first 4 feet of I-joists at the end of the bay.
3. For cantilevered I-joists, brace top and bottom flanges, and brace ends with closure panels, rim board, or cross-bridging.
4. Install and fully nail permanent sheathing to each I-joist before placing loads on the floor system. Then, stack building materials over beams or walls only.
5. Never install a damaged I-joist.

Improper storage or installation, failure to follow applicable building codes, failure to follow span ratings for Nordic I-joists, failure to follow allowable hole sizes and locations, or failure to use web stiffeners when required can result in serious accidents. Follow these installation guidelines carefully.

CHANTIERS CHIBOUGAMAU

PRODUCT WARRANTY

Chantiers Chibougamau guarantees that, in accordance with our specifications, Nordic products are free from manufacturing defects in material and workmanship.

Furthermore, Chantiers Chibougamau warrants that our products, when utilized in accordance with our handling and installation instructions, will meet or exceed our specifications for the lifetime of the structure.

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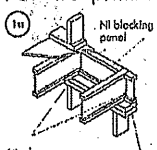
Document prepared for the use of Stephanie Gon from Alpa, Ontario. (Nordic Request 1810-095)

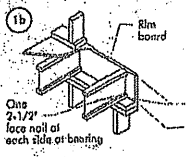
1a



Blocking Panel or Rim Joist	Maximum Factored Uniform Vertical Load* (plf)
NI Joists	3,300

*The uniform vertical load is limited to a joist depth of 16 inches or less and is based on standard term load duration. It shall not be used in the design of a bending member such as joist, header, or rafter. For concentrated vertical load transfer, see detail 1d.

1b



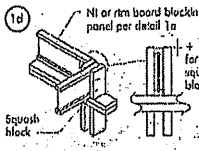
Blocking Panel or Rim Joist	Maximum Factored Uniform Vertical Load* (plf)
1-1/8" Rim Board Plus	8,090

*The uniform vertical load is limited to a rim board depth of 16 inches or less and is based on standard term load duration. It shall not be used in the design of a bending member, such as joist, header, or rafter. For concentrated vertical load transfer, see detail 1d.

- One 2-1/2" wire or spiral nail at top and bottom flange
- Attach rim board to top plate using 2-1/2" wire or spiral toe-nails at 6" o.c.
- To avoid splitting flange, start nails at least 1-1/2" from end of I-joist. Nails may be driven at an angle to avoid splitting at bearing plate.

Minimum bearing length shall be 1-3/4" for the end bearings, and 3-1/2" for the intermediate bearings when applicable.

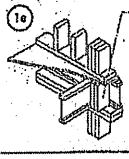
1d



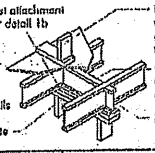
Pair of Squash Blocks	Maximum Factored Vertical Load per Pair of Squash Blocks (lbs)	
	3-1/2" wide	5-1/2" wide
2x Lumber	5,100	8,500
1-1/8" Rim Board Plus	4,200	6,600

Provide lateral bracing per detail 1a or 1b

1e



1g

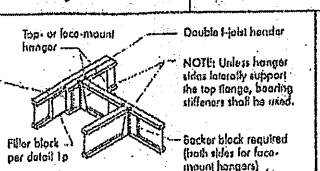


1h Backer block (use if hanger load exceeds 360 lbs). Before installing a backer block to a double I-joist, drive three additional 3" nails through the web and filler block where the backer block will fit. Clinch. Install backer tight to top flange. Use twelve 3" nails, clinched when possible. Maximum factored resistance for hanger for this detail = 1,620 lbs.

BACKER BLOCKS (Blocks must be long enough to permit required nailing without splitting)

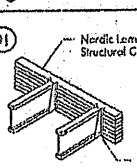
Flange Width	Material Thickness Required*	Minimum Depth**
2-1/2"	1"	5-1/2"
3-1/2"	1-1/2"	7-1/4"

* Minimum grade for backer block material shall be S-P-F No. 2 or better for solid sawn lumber and wood structural panels conforming to CAN/CSA-O325 or CAN/CSA-O437 Standard.
** For face-mount hangers use net joist depth minus 3-1/4" for joists with 1-1/2" thick flanges. For 2" thick flanges use net depth minus 4-1/4".



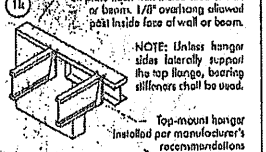
For hanger capacity see hanger manufacturer's recommendations. Verify double I-joist capacity to support concentrated loads.

1i

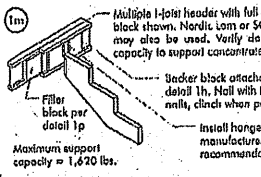


NOTE: Unless hanger sides laterally support the top flange, bearing stiffeners shall be used.

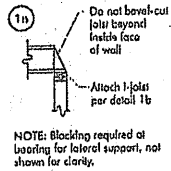
1k



1m

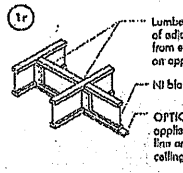


1n

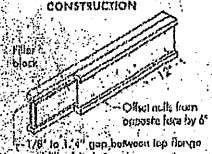


NOTE: Blocking required at bearing for lateral support, not shown for clarity.

1r



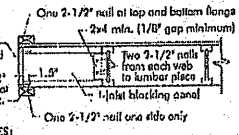
1p FILLER BLOCK REQUIREMENTS FOR DOUBLE I-JOIST CONSTRUCTION



NOTES:
1. Support back of I-joist web during nailing to prevent damage to web/flange connection.
2. Leave a 1/8 to 1/4-inch gap between top of filler block and bottom of top I-joist flange.
3. Filler block is required between joists for full length of span.
4. Nail joists together with two rows of 3" nails at 12 inches o.c. (clinched when possible) on each side of the double I-joist. Total of four nails per foot required. If nails can be clinched, only two nails per foot are required.
5. The maximum factored load that may be applied to one side of the double joist using this detail is 860 lb/ft. Verify double I-joist capacity.

Flange Size	Nail Depth	Filler Block Size
2-1/2" x 1-1/2"	9-1/2"	2-1/8" x 6"
	11-7/8"	2-1/8" x 8"
	14"	2-1/8" x 10"
	16"	2-1/8" x 12"
3-1/2" x 1-1/2"	9-1/2"	3" x 6"
	11-7/8"	3" x 8"
	14"	3" x 10"
	16"	3" x 12"
3-1/2" x 2"	11-7/8"	3" x 7"
	14"	3" x 9"
	16"	3" x 11"

1s



NOTES:
- In some local codes, blocking is prescriptively required in the first joist space (or first and second joist space) next to the starter joist. Where required, use local code requirements for spacing of the blocking.
- All nails are common spiral in this detail.

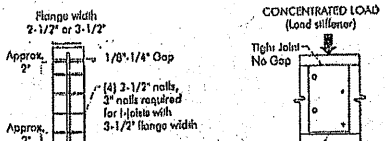
All nails shown in the above details are assumed to be common wire nails unless otherwise noted. 3" (0.122" dia.) common spiral nails may be substituted for 2-1/2" (0.120" dia.) common wire nails. Framing lumber assumed to be spruce-pine-fir No. 2 or better. Individual components not shown to scale for clarity.

WEB STIFFENERS

RECOMMENDATIONS:

- A *bearing stiffener* is required in all engineered applications with factored reactions greater than shown in the I-joist properties table found of the I-joist Construction Guide (C101). The gap between the stiffener and the flange is at the top.
- A *bearing stiffener* is required when the I-joist is supported in a hanger and the sides of the hanger do not extend up to, and support, the top flange. The gap between the stiffener and flange is at the top.
- A *load stiffener* is required at locations where a factored concentrated load greater than 2,370 lbs is applied to the top flange between supports, or in the case of a cantilever, anywhere between the cantilever tip and the support. These values are for standard term load duration, and may be adjusted for other load durations as permitted by the code. The gap between the stiffener and the flange is at the bottom.

FIGURE 2
WEB STIFFENER INSTALLATION DETAILS



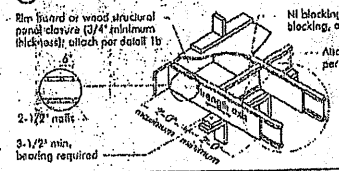
See the adjacent table for web stiffener size requirements

STIFFENER SIZE REQUIREMENTS

Flange Width	Web Stiffener Size Each Side of Web
2-1/2"	1" x 2-5/16" minimum width
3-1/2"	1-1/2" x 2-5/16" minimum width

CANTILEVER DETAILS FOR VERTICAL BUILDING OFFSET

4a Method 1 — SHEATHING REINFORCEMENT ONE SIDE



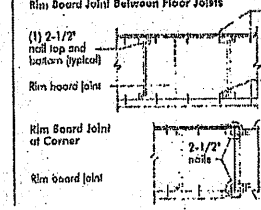
Method 2 — SHEATHING REINFORCEMENT TWO SIDES

Use same installation as Method 1 but reinforce both sides of I-joist with sheathing.

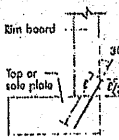
NOTE: Canadian softwood plywood sheathing or equivalent (minimum thickness 3/4") required on sides of joist. Depth shall match the full height of the joist. Nail with 2-1/2" nails at 6" o.c., top and bottom flange. Install with face grain horizontal. Attach I-joist to plate at all supports per detail 1b. Verify reinforced I-joist capacity.

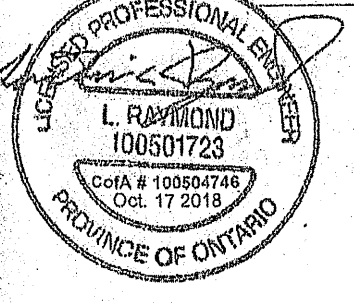
RIM BOARD INSTALLATION DETAILS

8a ATTACHMENT DETAILS WHERE RIM BOARDS ABUT



8b TOE-NAIL CONNECTION AT RIM BOARD





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