

JT/PL: 45147/105729 LI: 318278(290673)

Builder: Gold Park

Project: Pine Valley

Location: Vaughan

Date: November 12, 2017

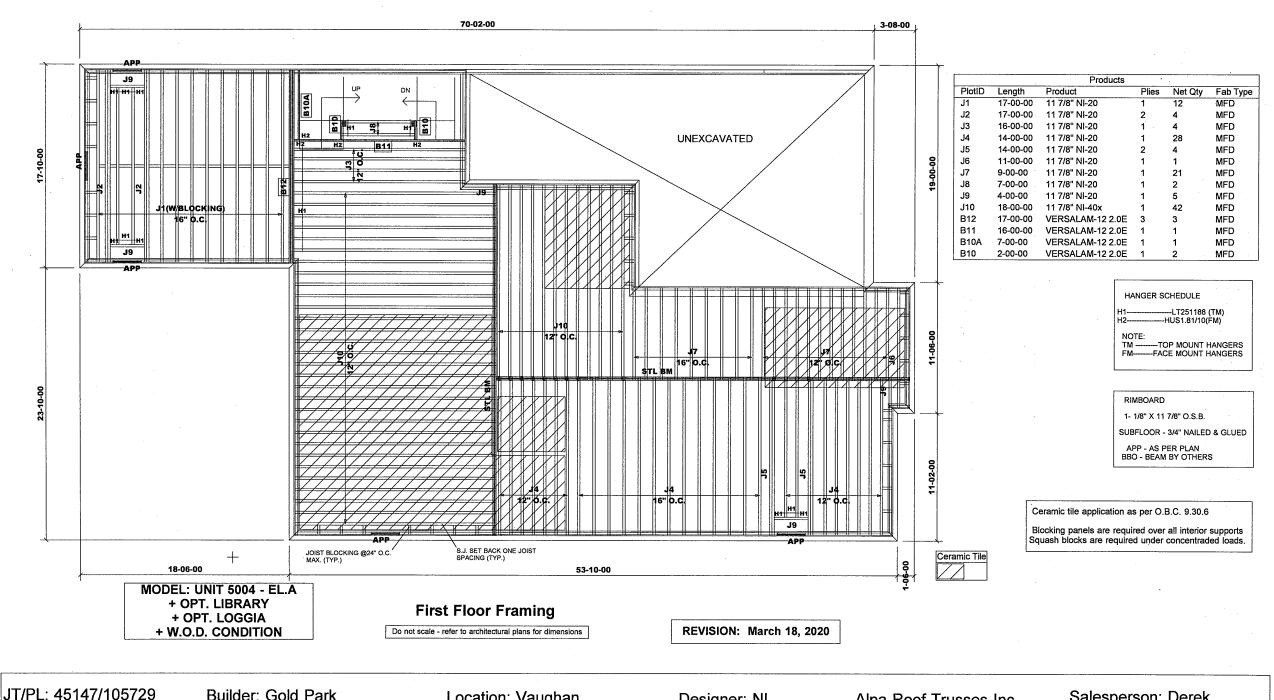
Designer: NL

Sheet: 1 of 26

Alpa Roof Trusses Inc.

Maple, Ontario

Salesperson: Derek



Builder: Gold Park

Project: Pine Valley

Location: Vaughan

Date: November 12, 2017

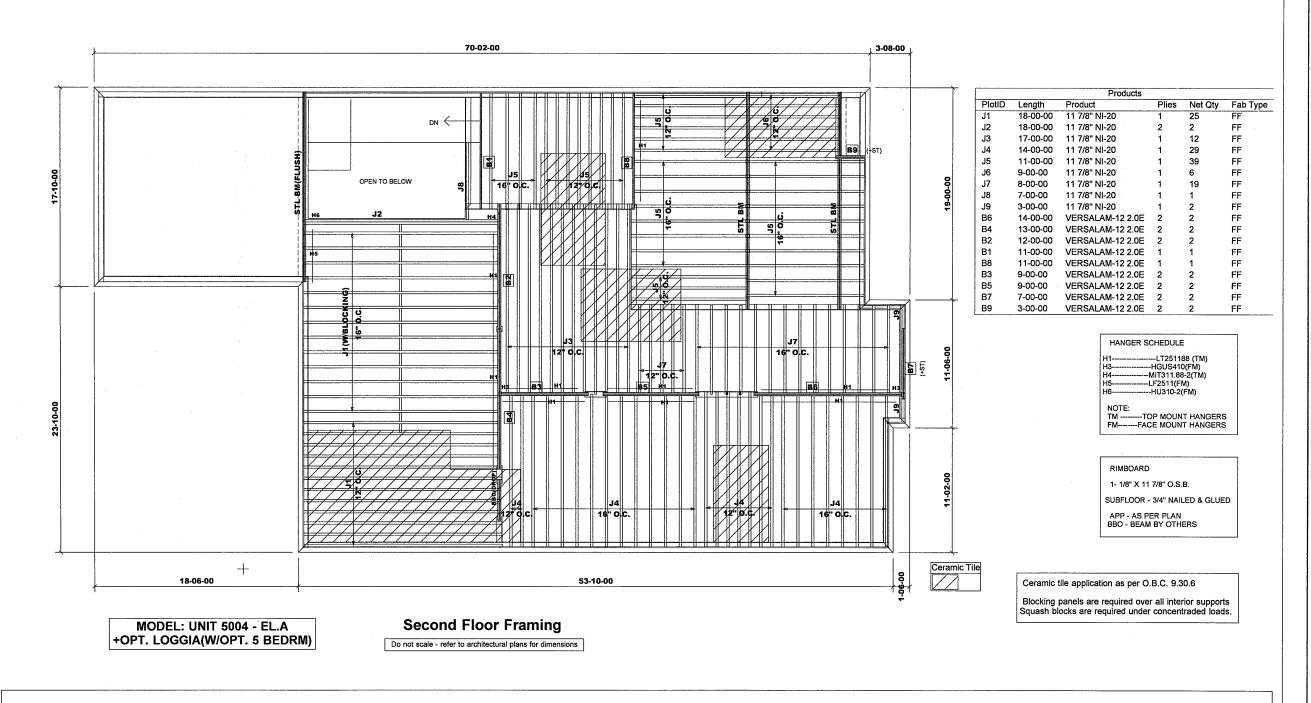
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Sheet: 2 of 26

Alpa Roof Trusses Inc.

Maple, Ontario

Salesperson: Derek



LI: 318278(290673)

Builder: Gold Park

Project: Pine Valley

Location: Vaughan

Date: November 15, 2017

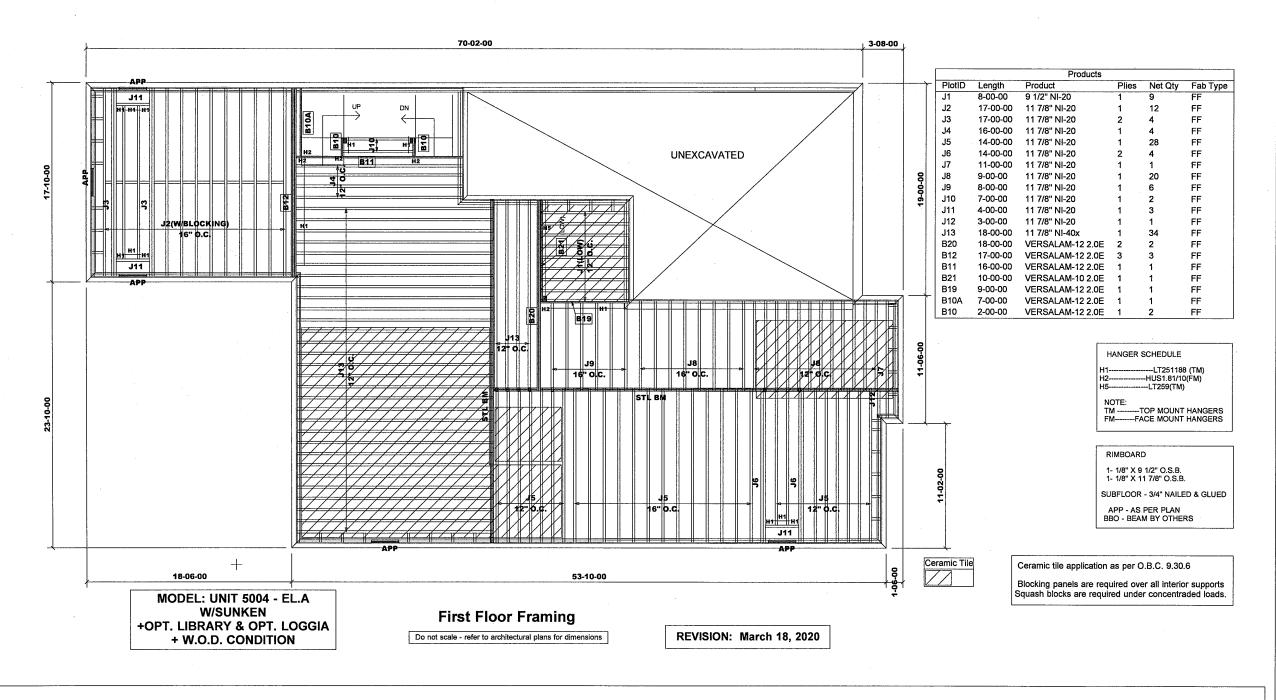
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Sheet: 3 of 26

Alpa Roof Trusses Inc.

Maple, Ontario

Salesperson: Derek



LI: 318278(290673)

Builder: Gold Park

Project: Pine Valley

Location: Vaughan

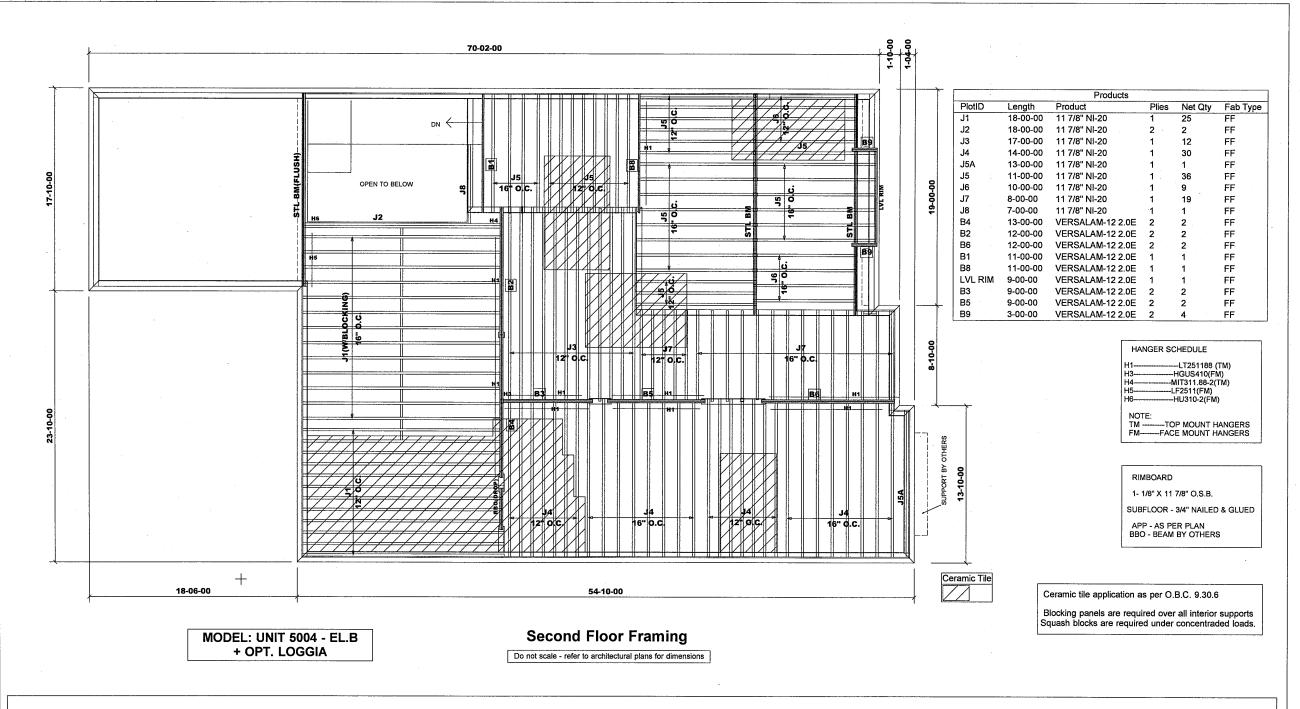
Date: November 15, 2017

Designer: NL

Sheet: 4 of 26

Alpa Roof Trusses Inc.
Maple, Ontario

Salesperson: Derek



LI: 318278(290673)

Builder: Gold Park

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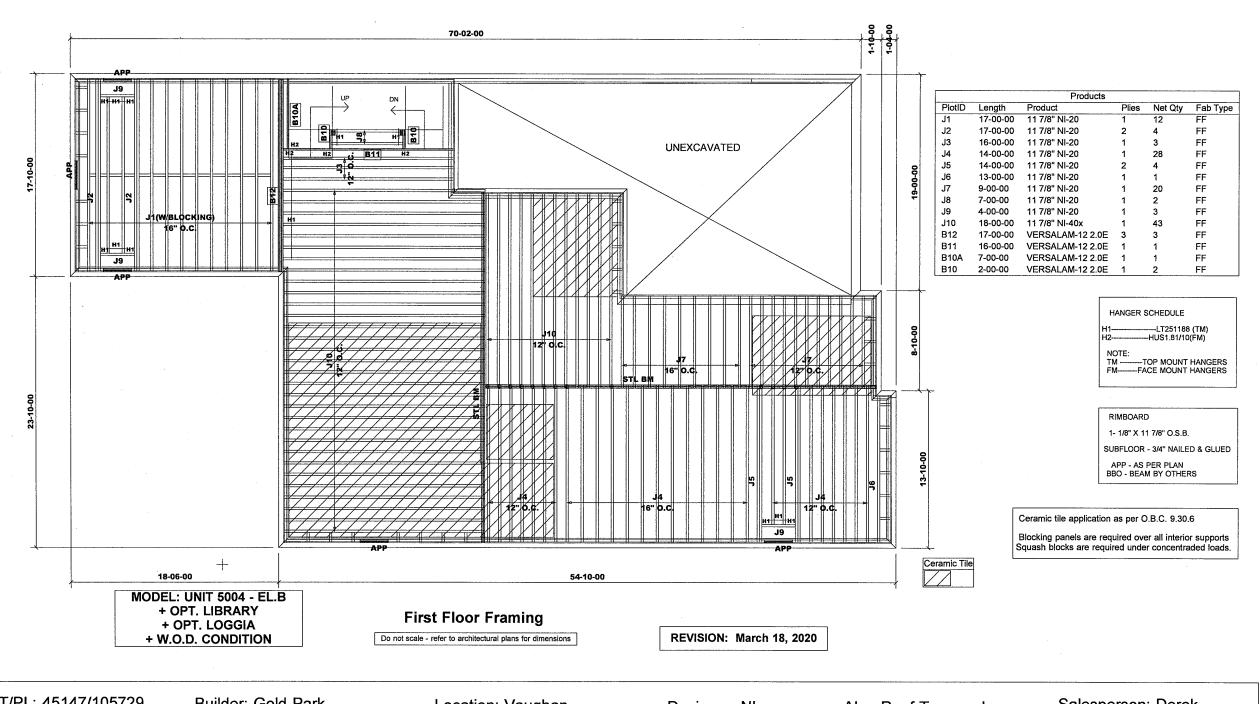
Date: November 12, 2017

Designer: NL

Sheet: 5 of 26

Alpa Roof Trusses Inc.
Maple, Ontario

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LI: 318278(290673)

Builder: Gold Park

Project: Pine Valley

Location: Vaughan

Date: November 12, 2017

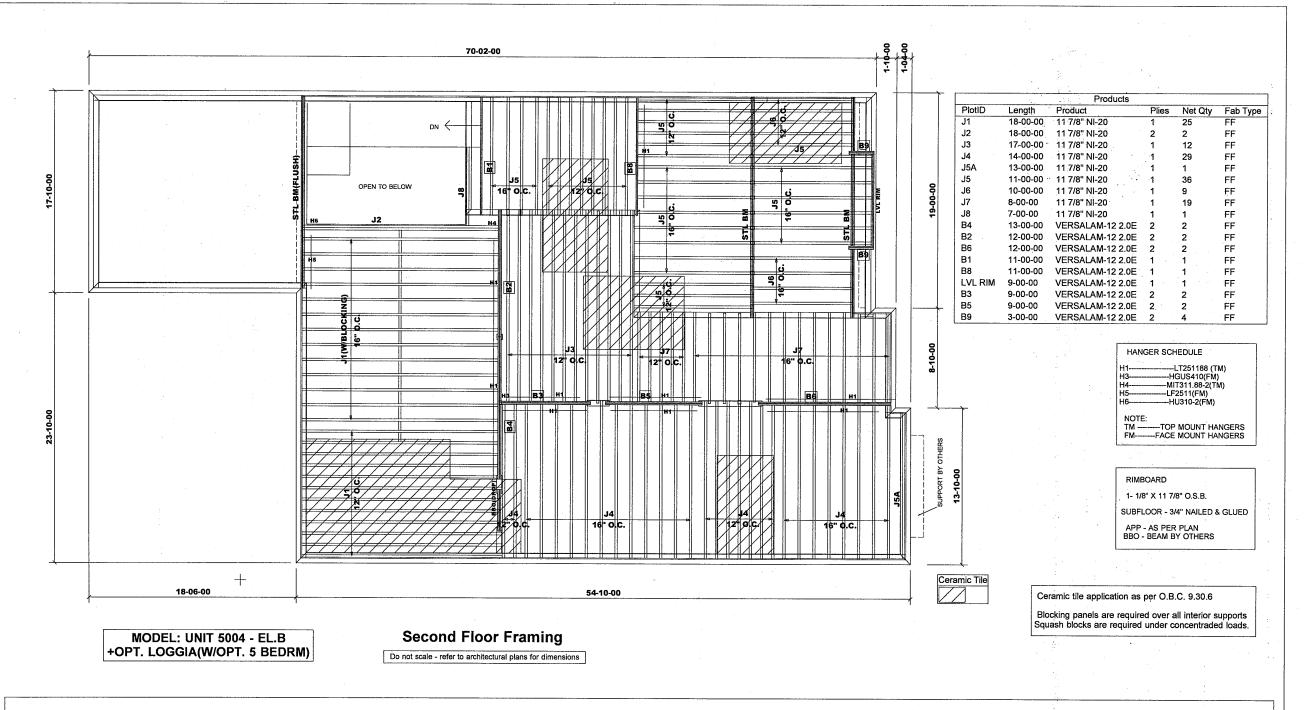
Designer: NL

Sheet: 6 of 26

Alpa Roof Trusses Inc.

Maple, Ontario

Salesperson: Derek



LI: 318278(290673)

Builder: Gold Park

Project: Pine Valley

Location: Vaughan

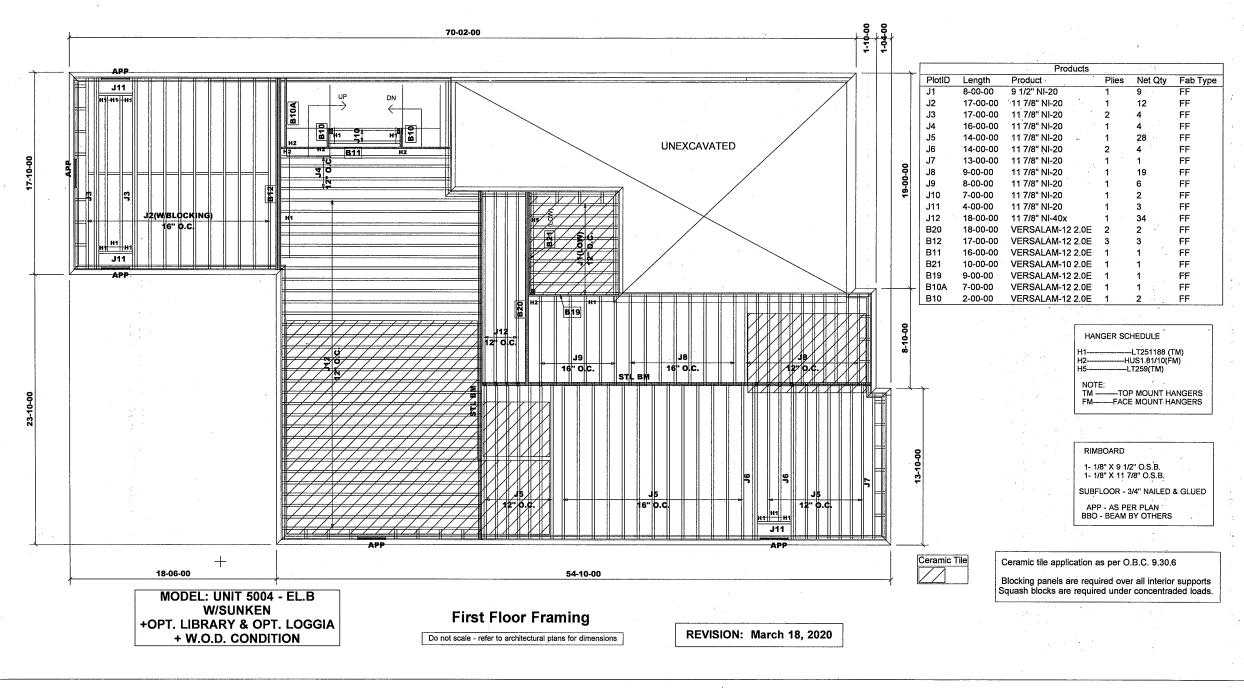
Date: November 15, 2017

Designer: NL

Sheet: 7 of 26

Alpa Roof Trusses Inc.
Maple, Ontario

Salesperson: Derek



LI: 318278(290673)

Builder: Gold Park

Project: Pine Valley

Location: Vaughan

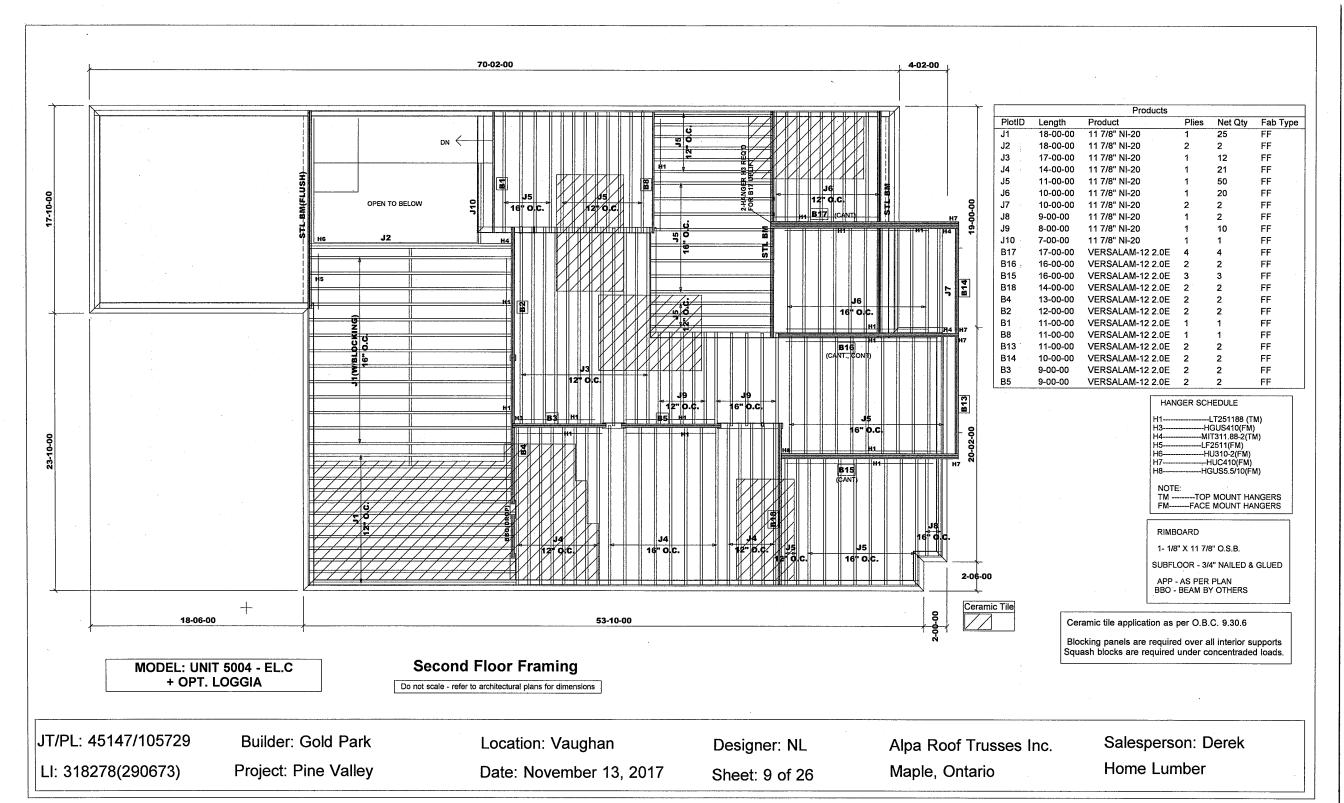
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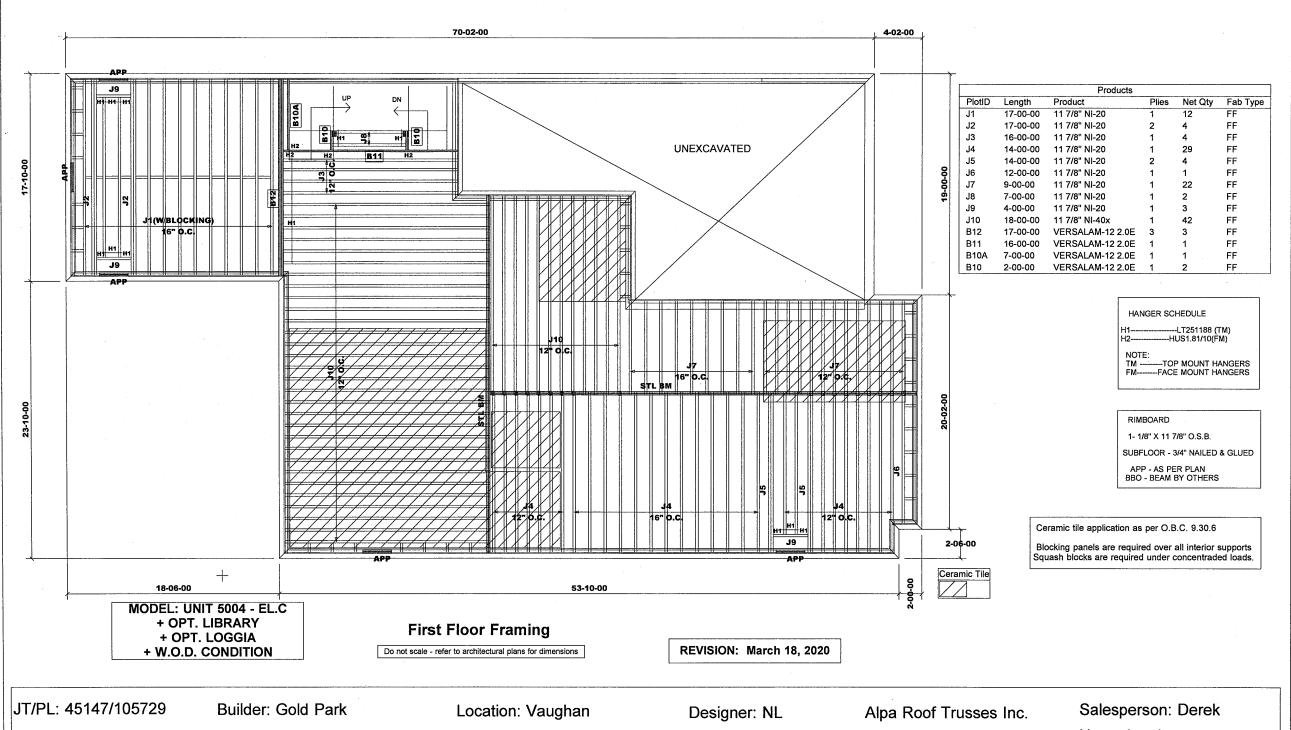
Designer: NL

Sheet: 8 of 26

Alpa Roof Trusses Inc.
Maple, Ontario

Salesperson: Derek



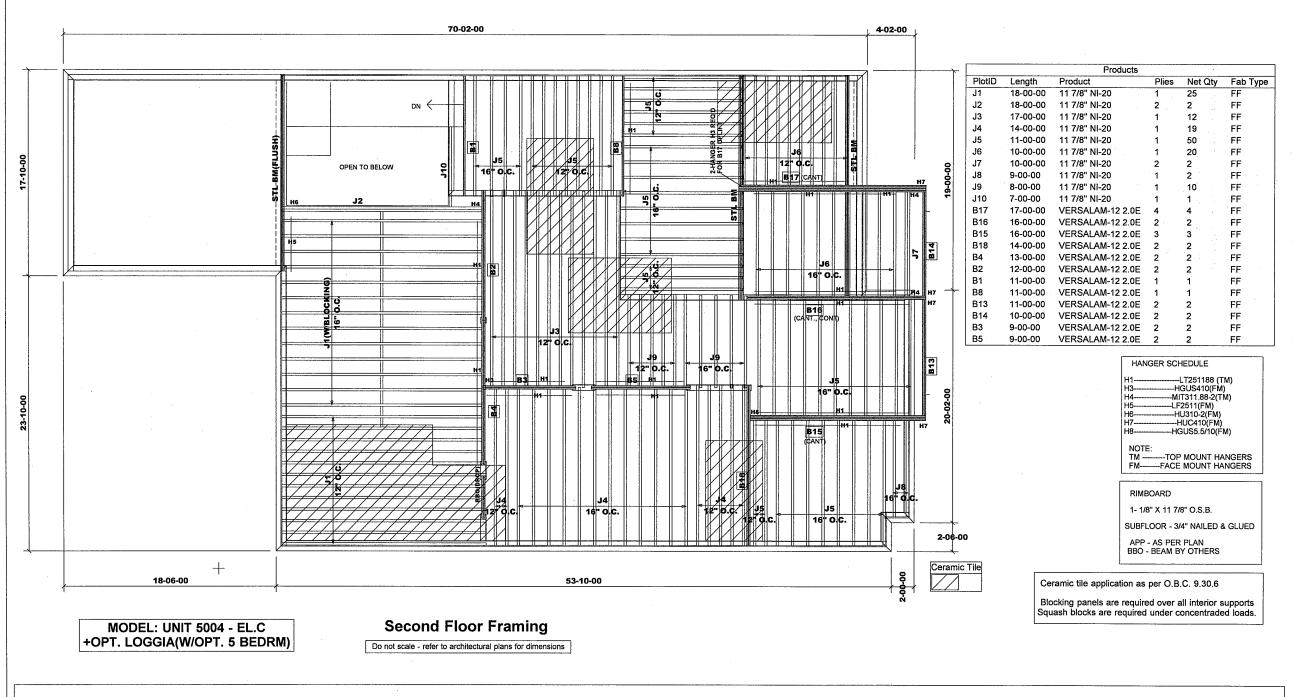


Project: Pine Valley

Date: November 13, 2017

Sheet: 10 of 26

Maple, Ontario



LI: 318278(290673)

Builder: Gold Park

Project: Pine Valley

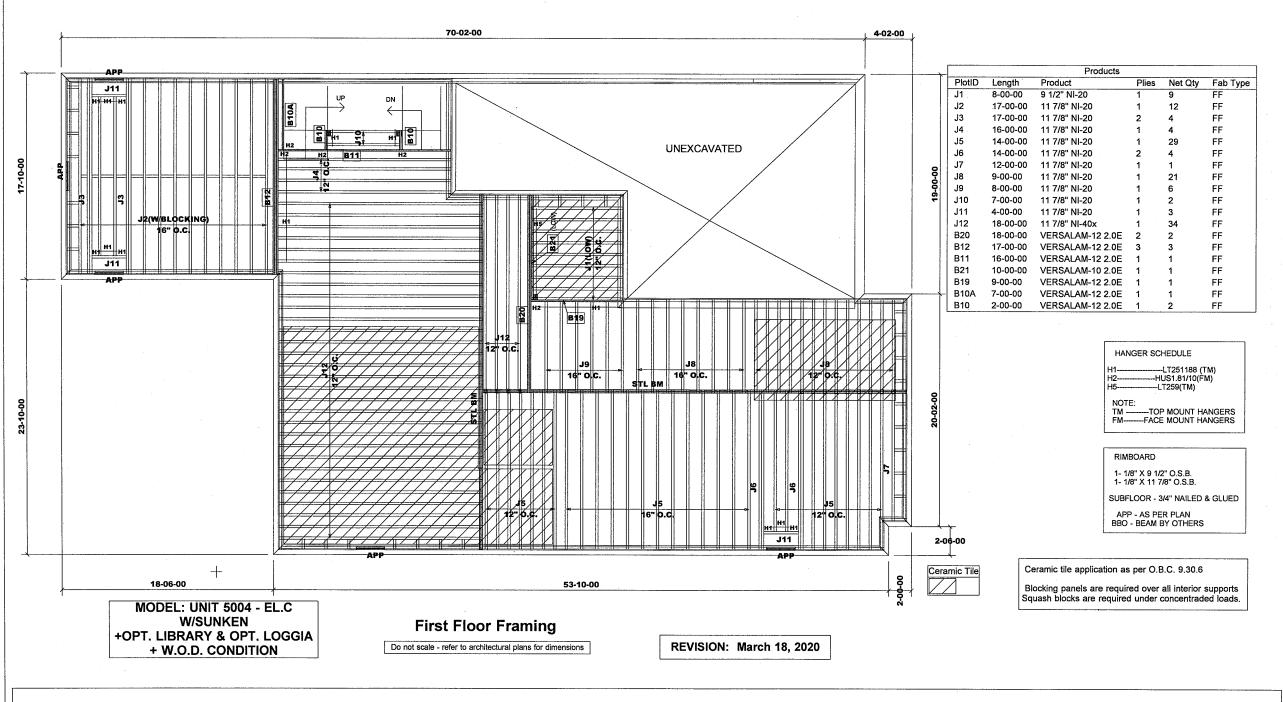
Location: Vaughan

Date: November 15, 2017

Designer: NL

Sheet: 11 of 26

Alpa Roof Trusses Inc. Maple, Ontario Salesperson: Derek



LI: 318278(290673)

Builder: Gold Park

Project: Pine Valley

Location: Vaughan

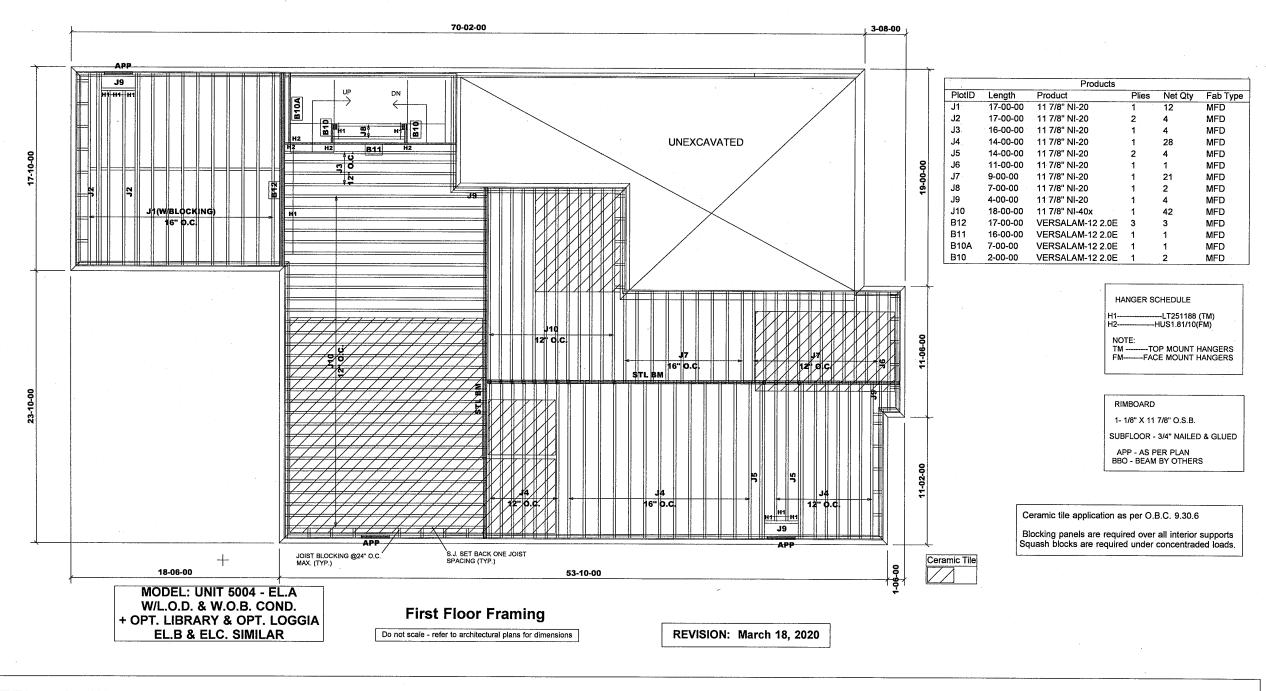
Date: November 15, 2017

Designer: NL

Sheet: 12 of 26

Alpa Roof Trusses Inc.
Maple, Ontario

Salesperson: Derek



LI: 318278(290673)

Builder: Gold Park

Project: Pine Valley

Location: Vaughan

Date: November 12, 2017

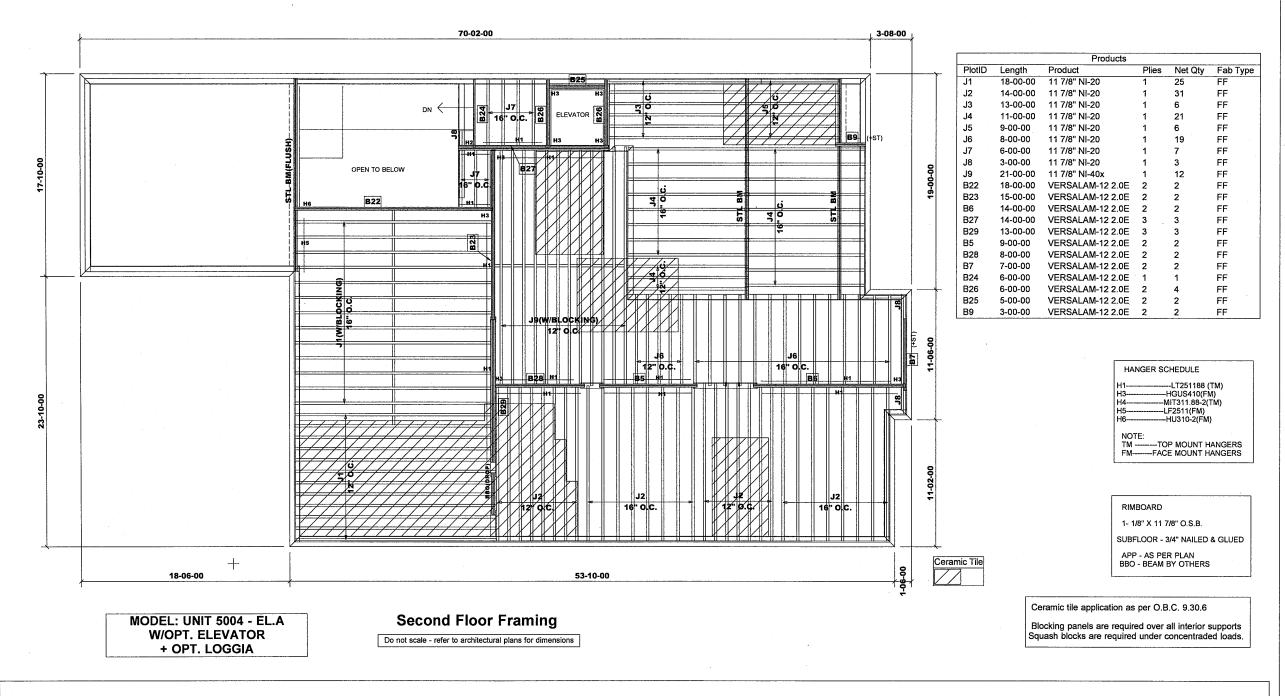
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Sheet: 13 of 26

Alpa Roof Trusses Inc.

Maple, Ontario

Salesperson: Derek



LI: 318278(290673)

Builder: Gold Park

Project: Pine Valley

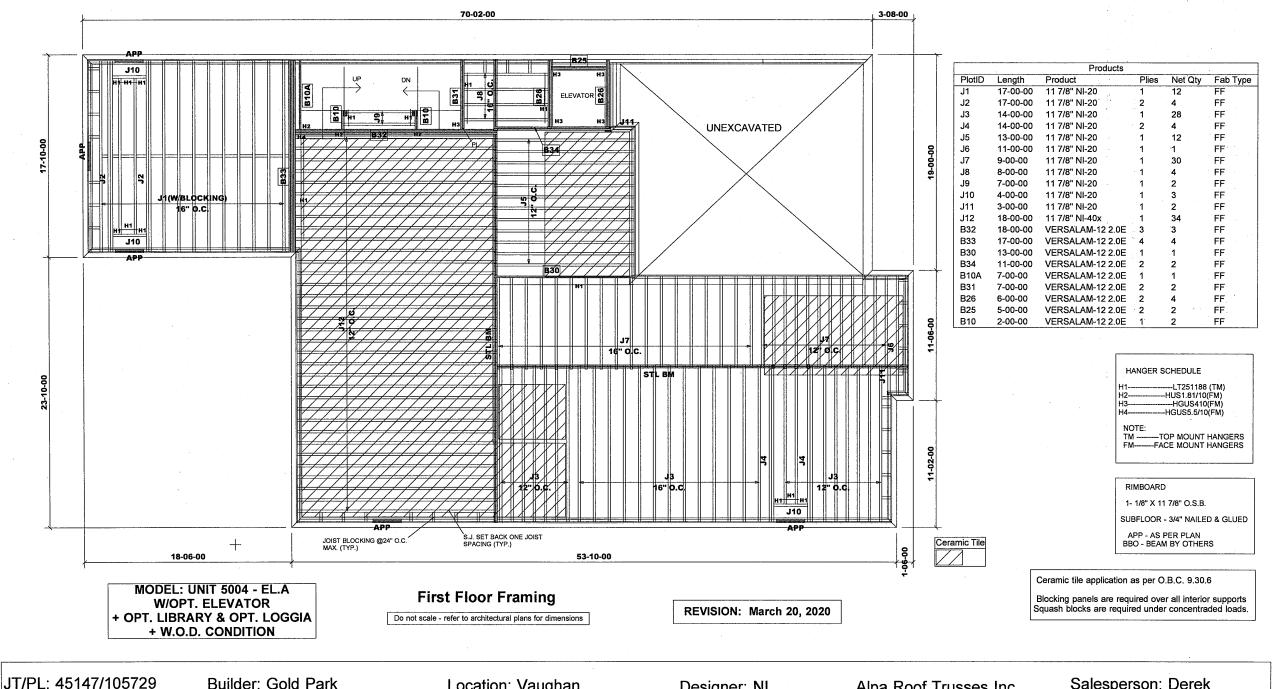
Location: Vaughan

Date: November 16, 2017

Designer: NL

Sheet: 14 of 26

Alpa Roof Trusses Inc. Maple, Ontario Salesperson: Derek



Builder: Gold Park

Project: Pine Valley

Location: Vaughan

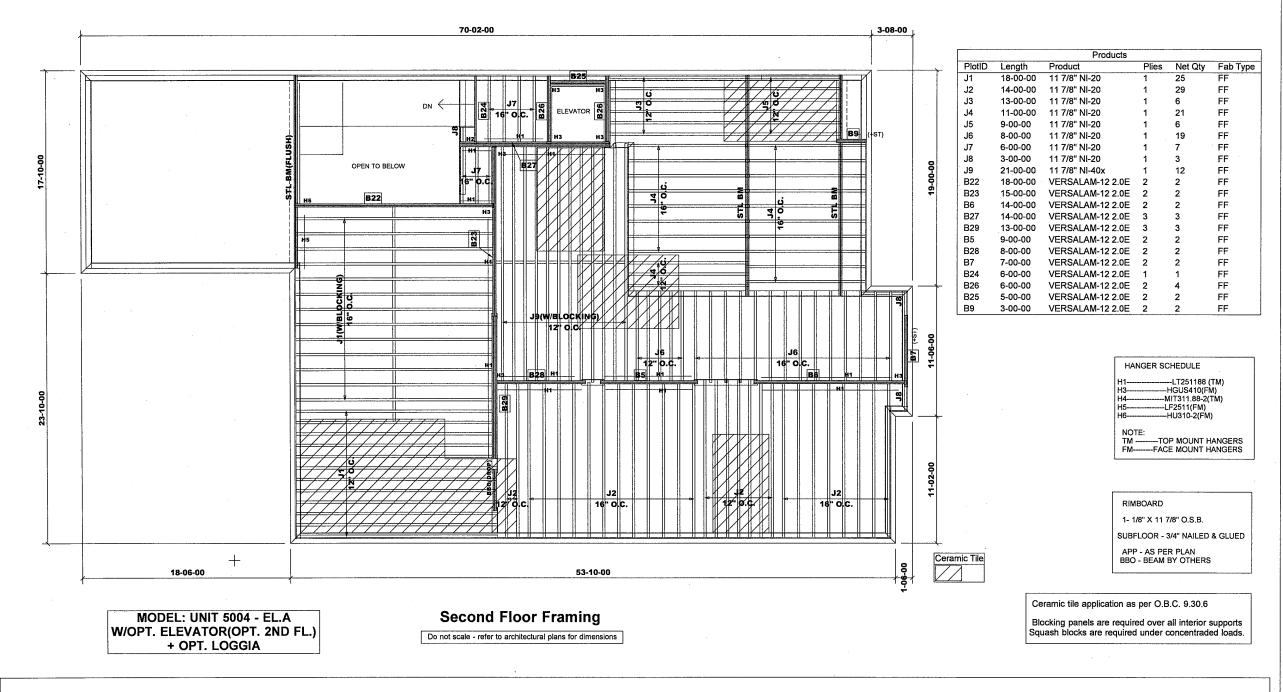
Date: November 16, 2017

Designer: NL

Sheet: 15 of 26

Alpa Roof Trusses Inc. Maple, Ontario

Salesperson: Derek



JT/PL: 45147/105729 LI: 318278(290673)

Builder: Gold Park

Project: Pine Valley

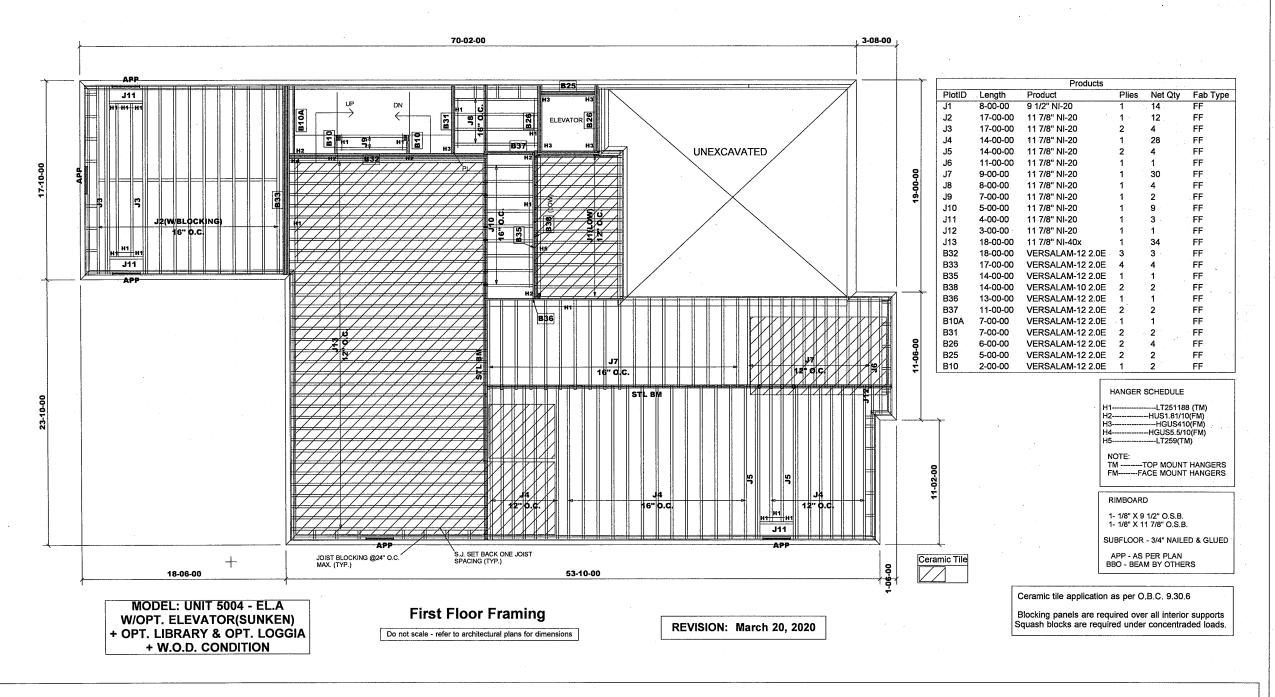
Location: Vaughan

Date: November 16, 2017

Designer: NL

Sheet: 16 of 26

Alpa Roof Trusses Inc. Maple, Ontario Salesperson: Derek



LI: 318278(290673)

Builder: Gold Park

Project: Pine Valley

Location: Vaughan

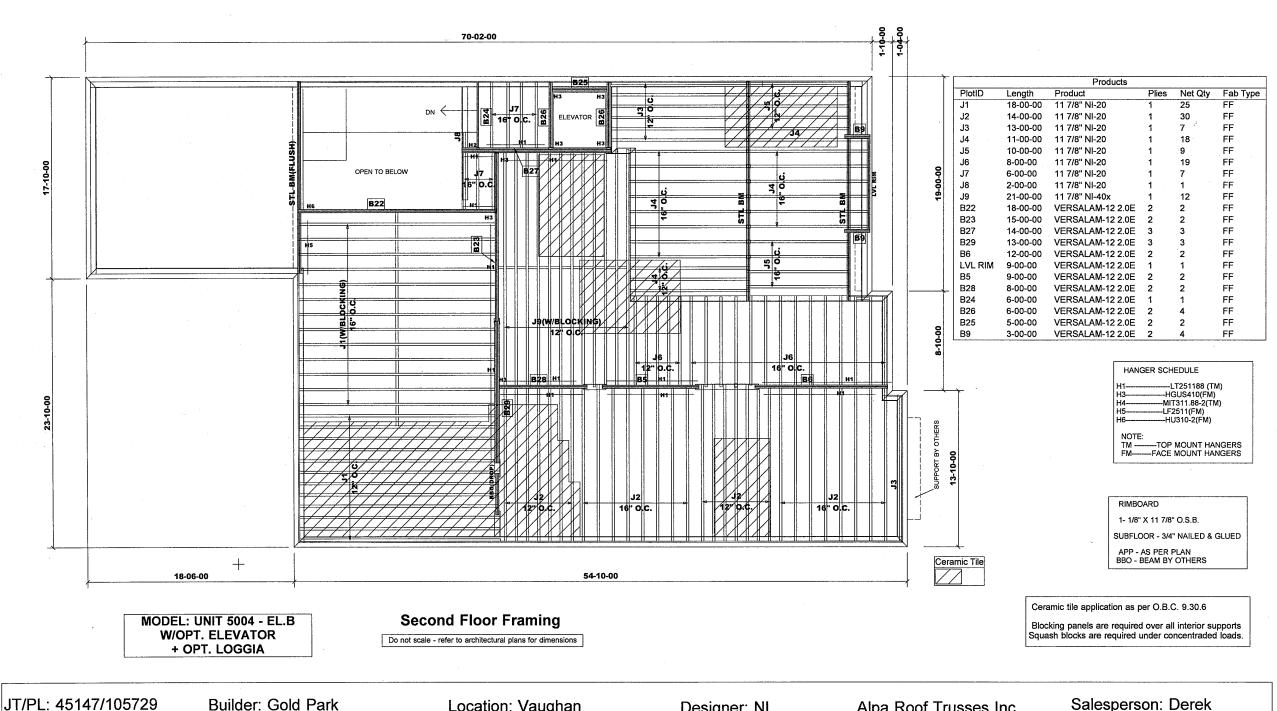
Date: November 16, 2017

Designer: NL

Sheet: 17 of 26

Alpa Roof Trusses Inc.
Maple, Ontario

Salesperson: Derek



Project: Pine Valley

Location: Vaughan

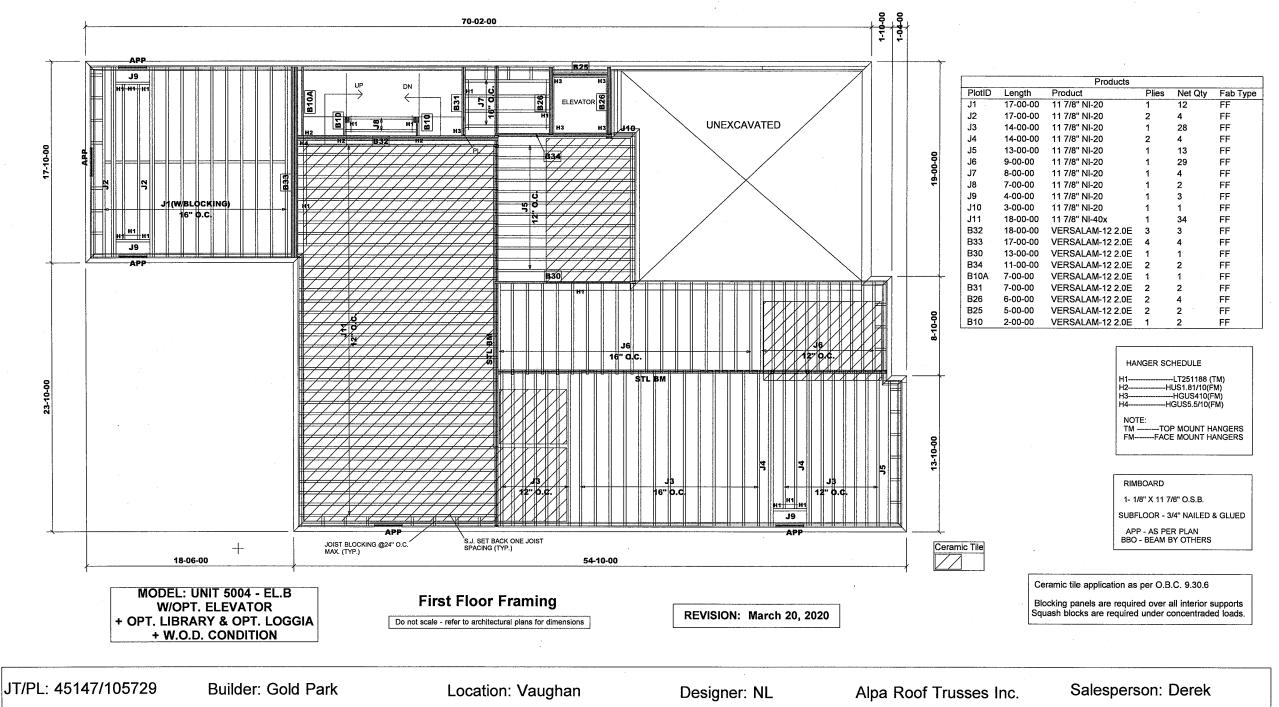
Date: November 16, 2017

Designer: NL

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Alpa Roof Trusses Inc. Maple, Ontario

Salesperson: Derek

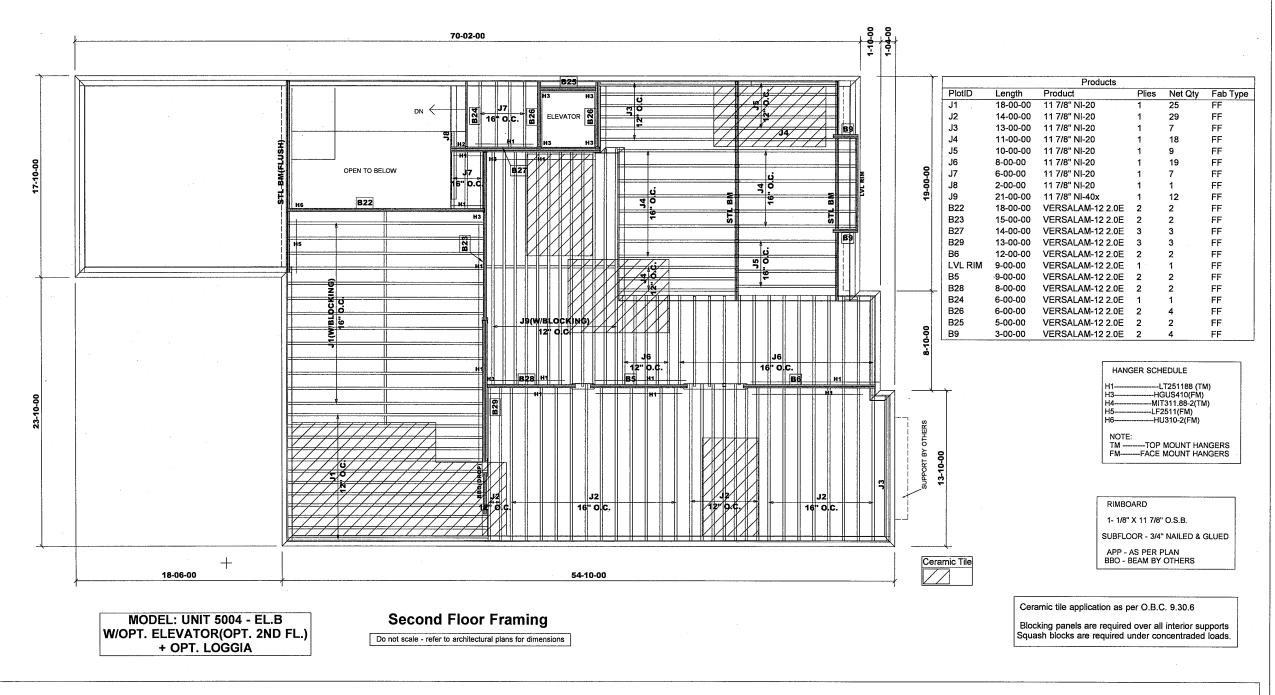


Project: Pine Valley

Date: November 16, 2017

Sheet: 19 of 26

Maple, Ontario



LI: 318278(290673)

Builder: Gold Park

Project: Pine Valley

Location: Vaughan

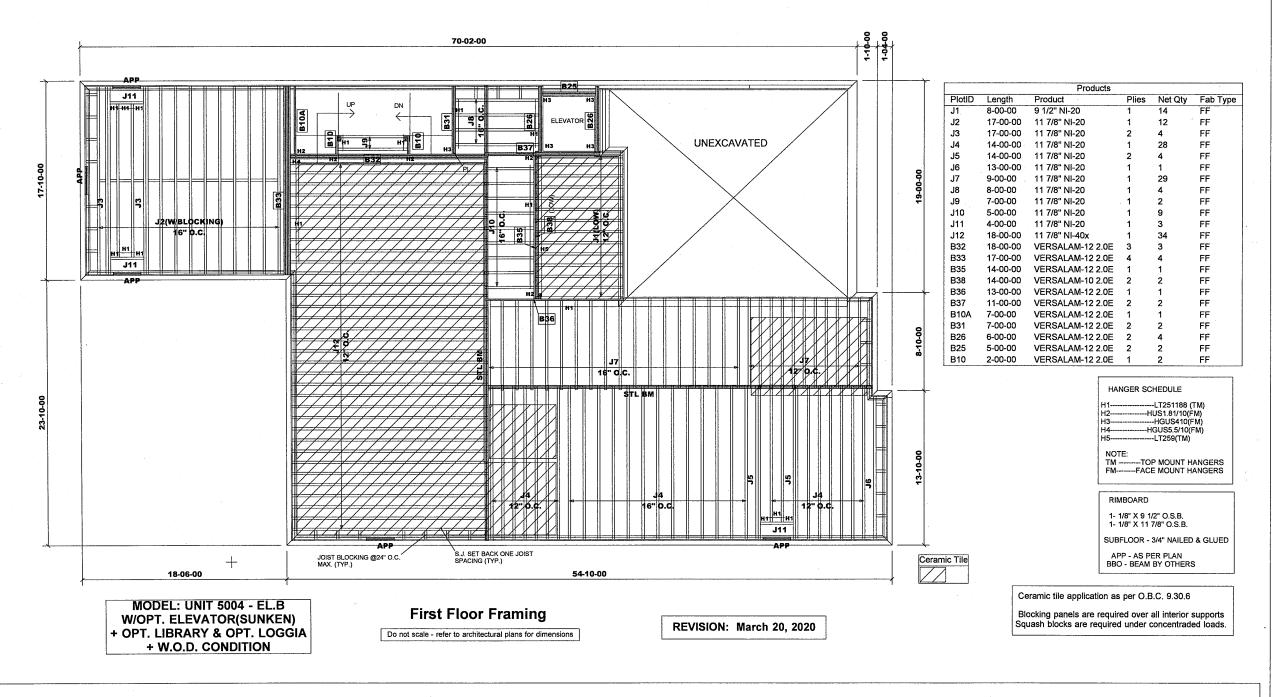
Date: November 16, 2017

Designer: NL

Sheet: 20 of 26

Alpa Roof Trusses Inc.
Maple, Ontario

Salesperson: Derek



LI: 318278(290673)

Builder: Gold Park

Project: Pine Valley

Location: Vaughan

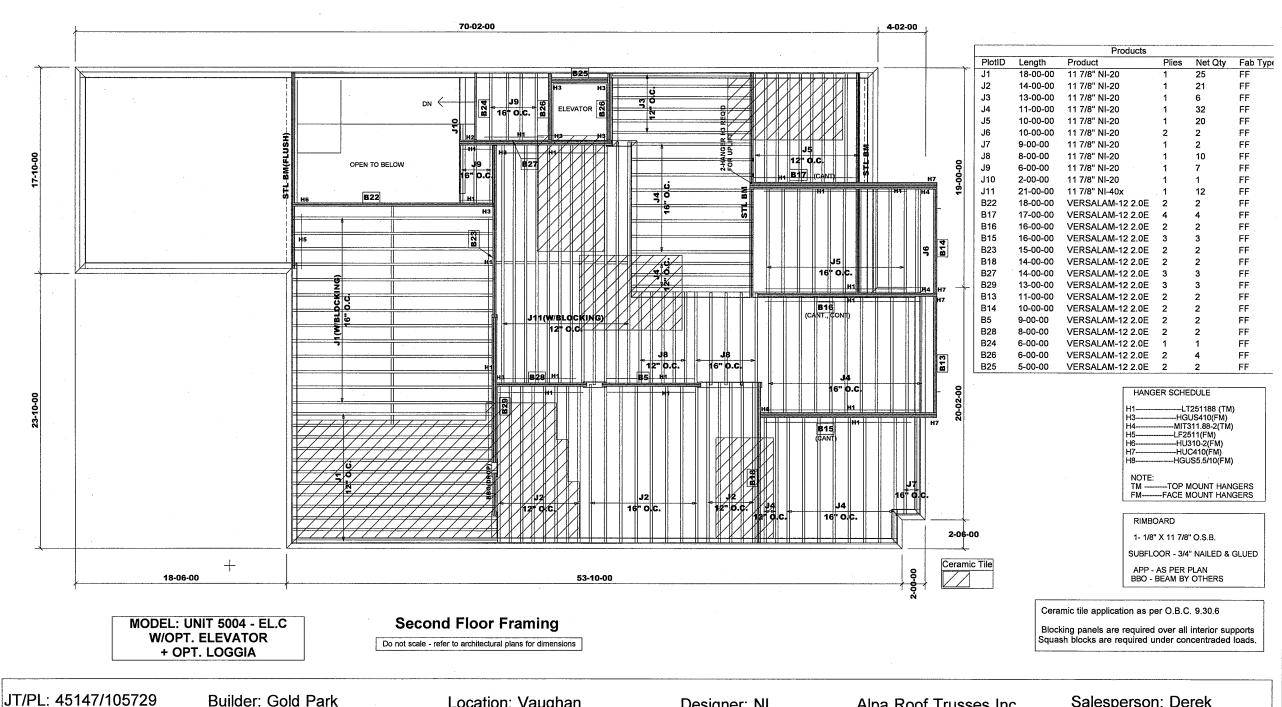
Date: November 16, 2017

Designer: NL

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Alpa Roof Trusses Inc.
Maple, Ontario

Salesperson: Derek



Project: Pine Valley

Location: Vaughan

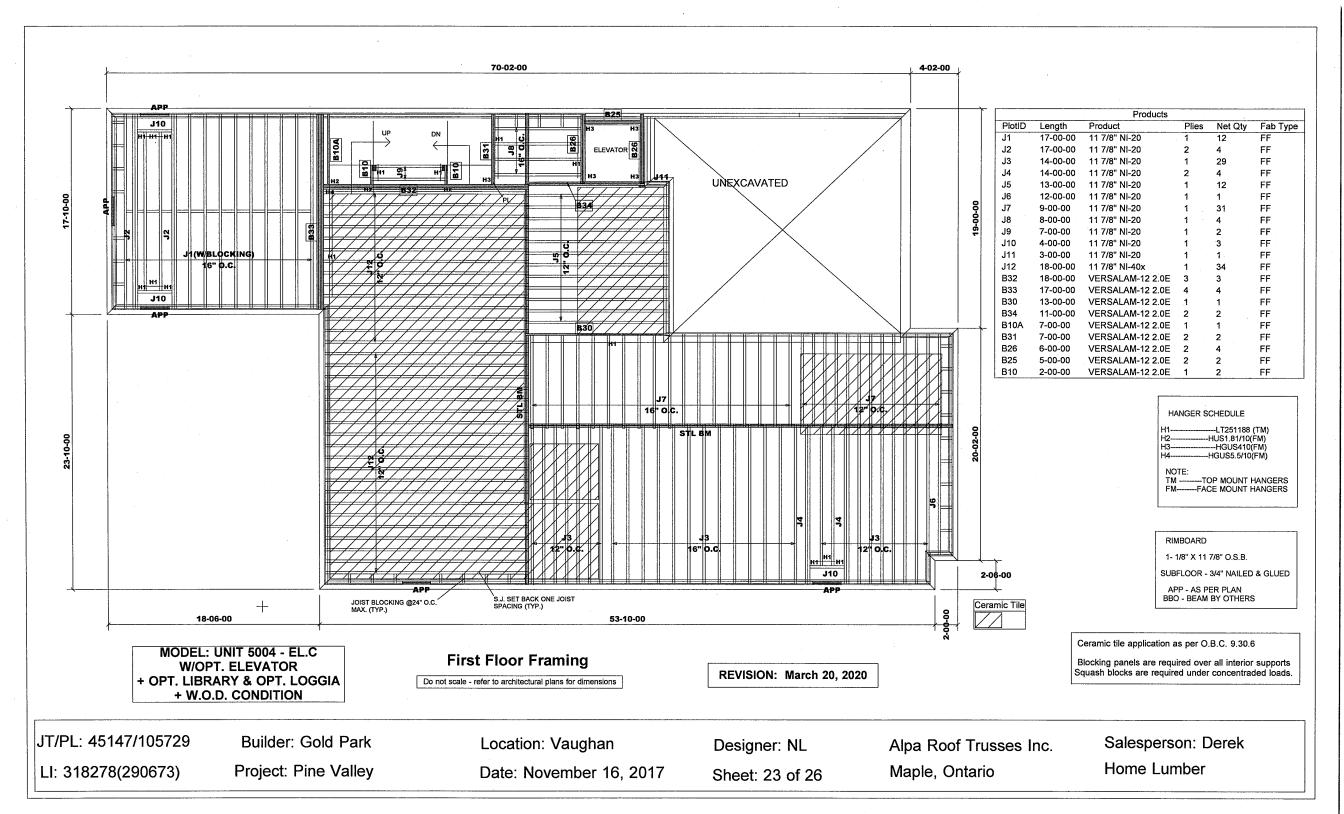
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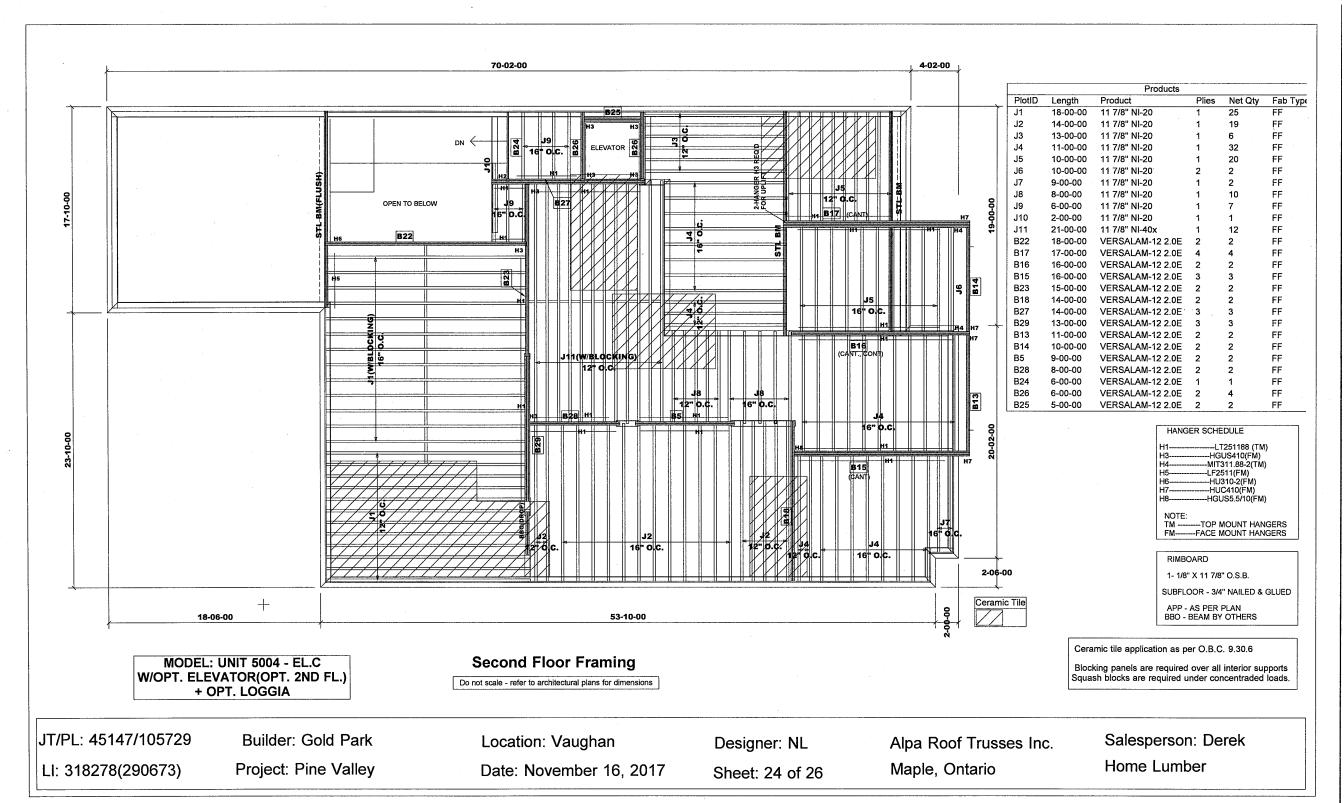
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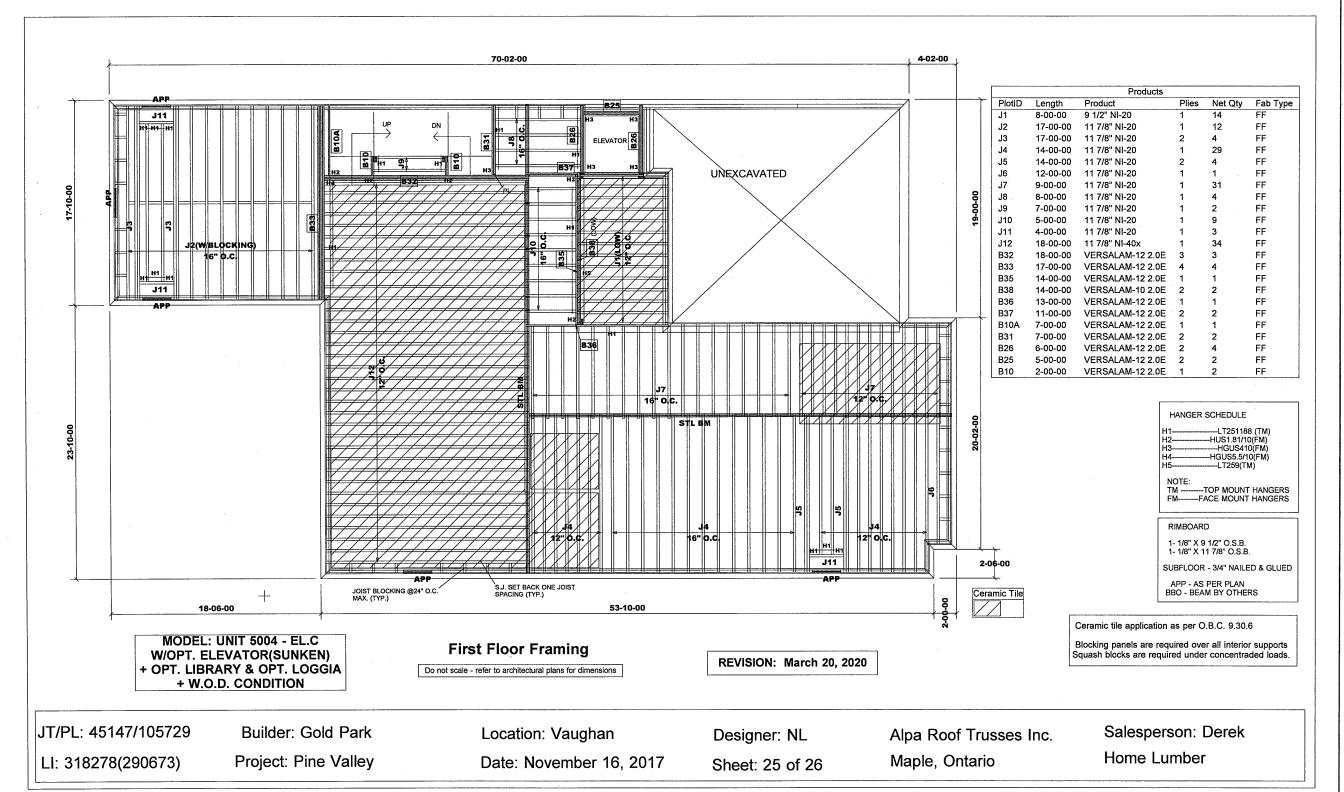
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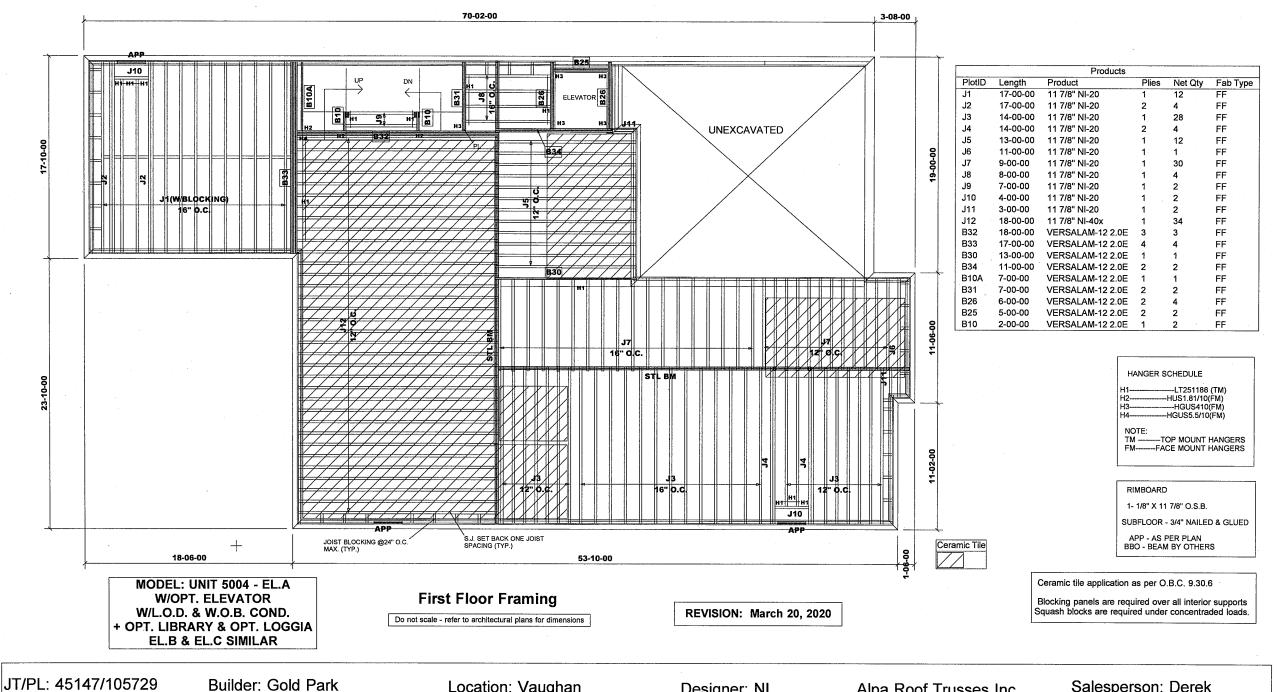
Alpa Roof Trusses Inc. Maple, Ontario

Salesperson: Derek









Project: Pine Valley

Location: Vaughan

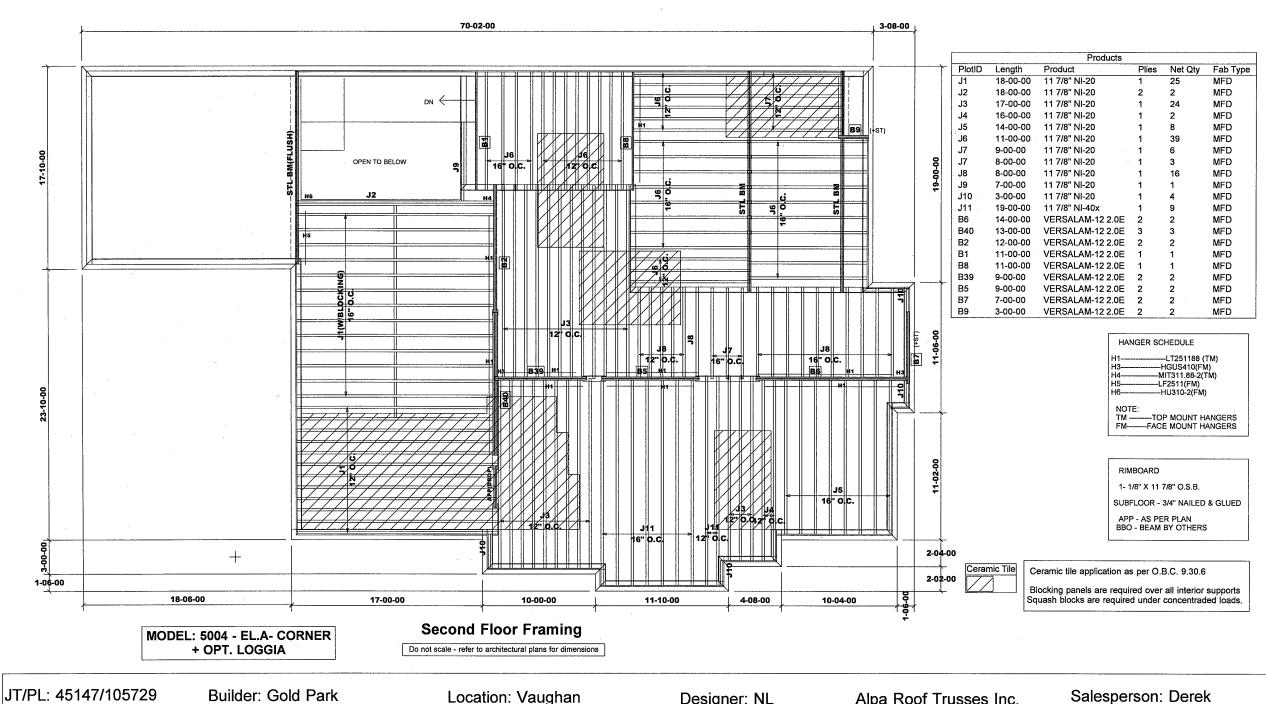
Date: November 16, 2017

Designer: NL

Sheet: 26 of 26

Alpa Roof Trusses Inc. Maple, Ontario

Salesperson: Derek



Project: Pine Valley

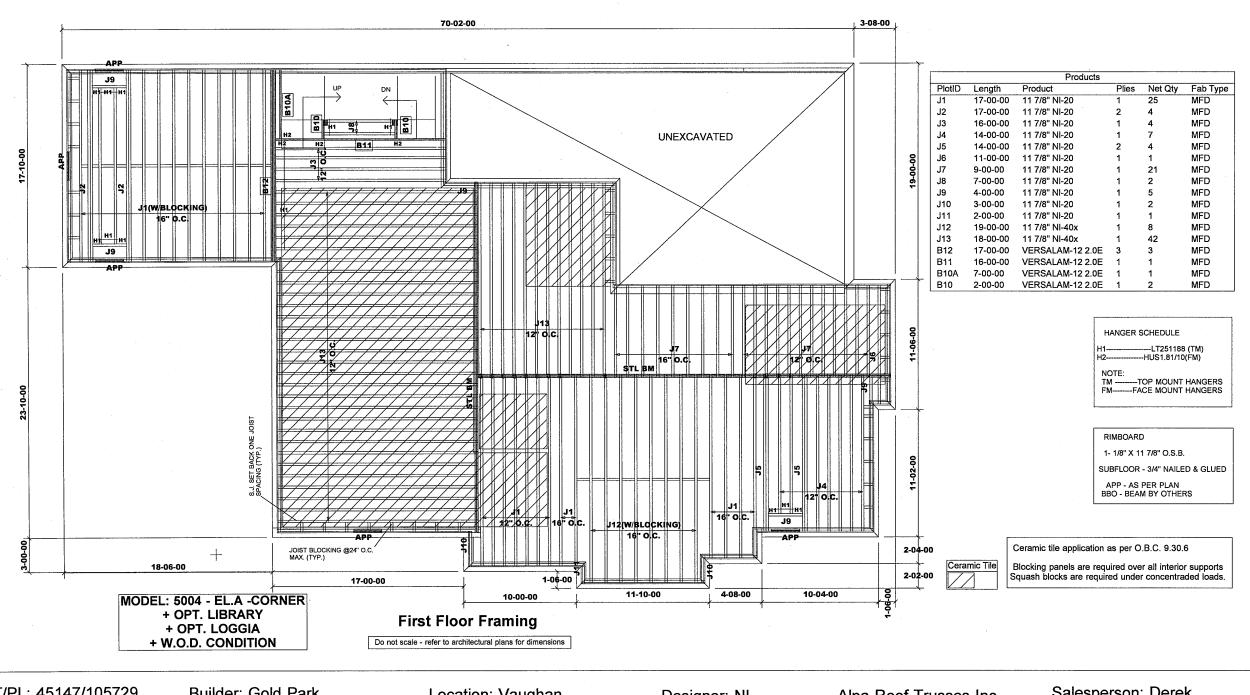
Location: Vaughan

Date: March 20, 2020

Designer: NL

Sheet: 1 of 10

Alpa Roof Trusses Inc. Maple, Ontario



LI: 318278(290673)

Builder: Gold Park

Project: Pine Valley

Location: Vaughan

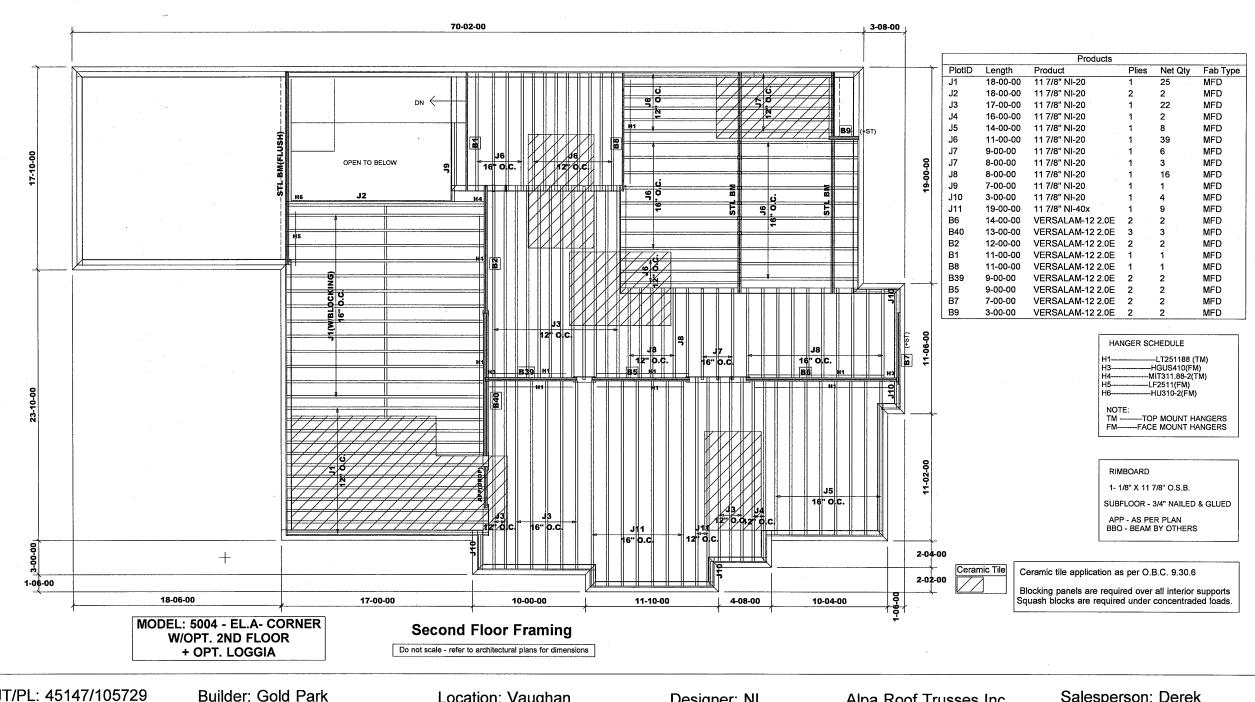
Date: March 20, 2020

Designer: NL

Sheet: 2 of 10

Alpa Roof Trusses Inc. Maple, Ontario

Salesperson: Derek



LI: 318278(290673)

Project: Pine Valley

Location: Vaughan

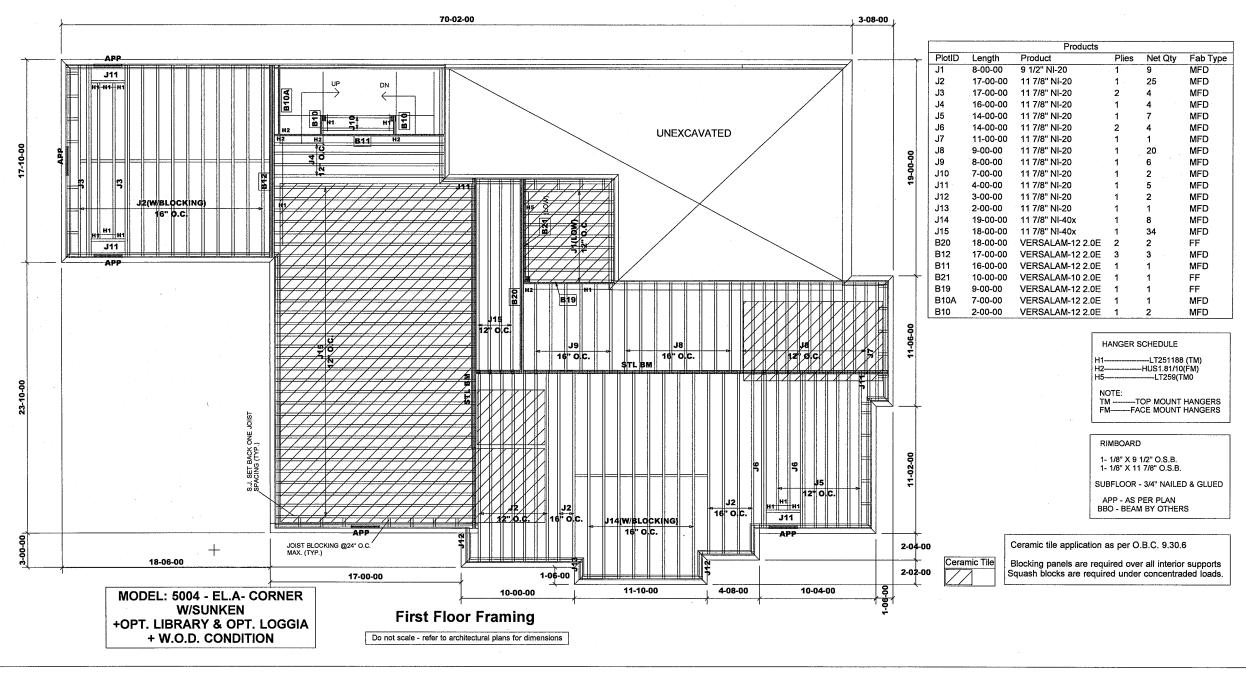
Date: March 20, 2020

Designer: NL

Sheet: 3 of 10

Alpa Roof Trusses Inc. Maple, Ontario

Salesperson: Derek



LI: 318278(290673)

Builder: Gold Park

Project: Pine Valley

Location: Vaughan

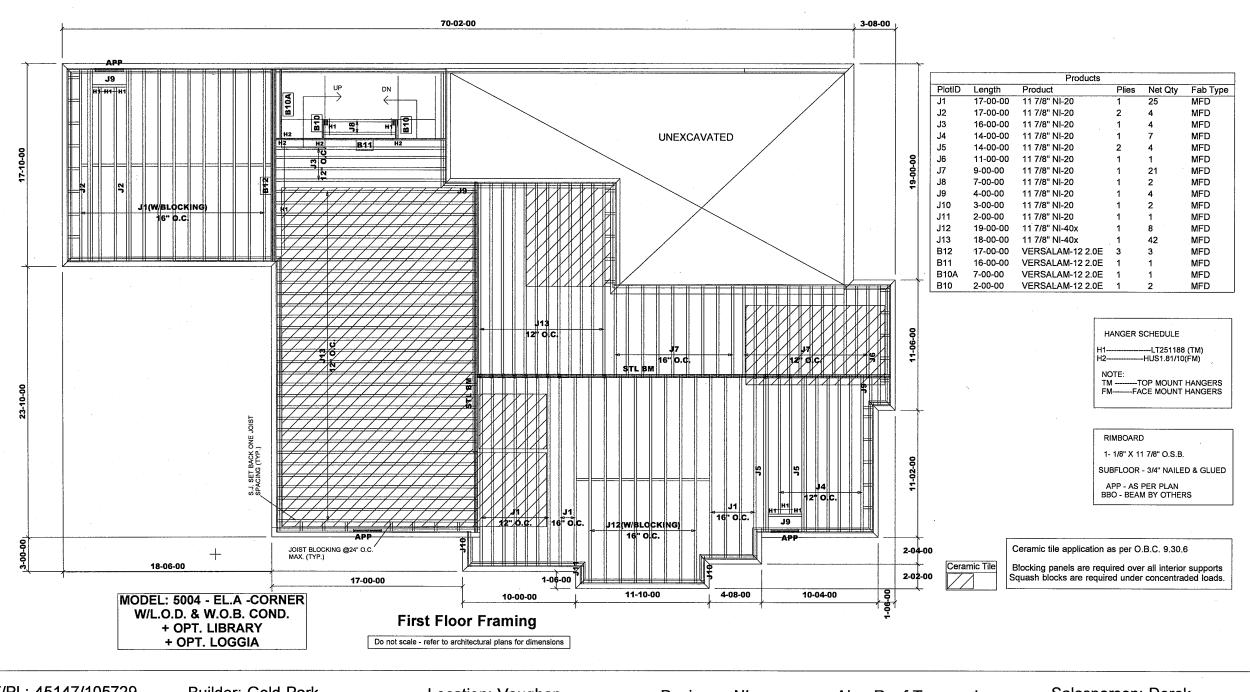
Date: March 20, 2020

Designer: NL

Sheet: 4 of 10

Alpa Roof Trusses Inc.
Maple, Ontario

Salesperson: Derek



LI: 318278(290673)

Builder: Gold Park

Project: Pine Valley

Location: Vaughan

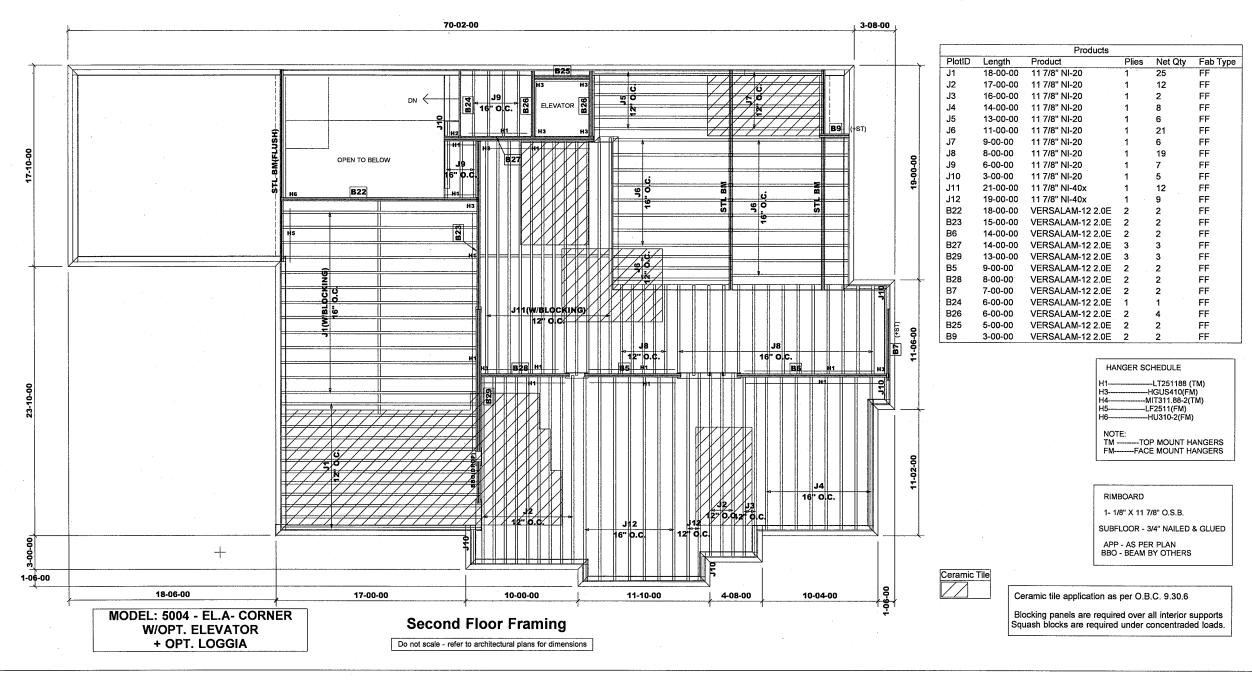
Date: March 20, 2020

Designer: NL

Sheet: 5 of 10

Alpa Roof Trusses Inc.
Maple, Ontario

Salesperson: Derek



LI: 318278(290673)

Builder: Gold Park

Project: Pine Valley

Location: Vaughan

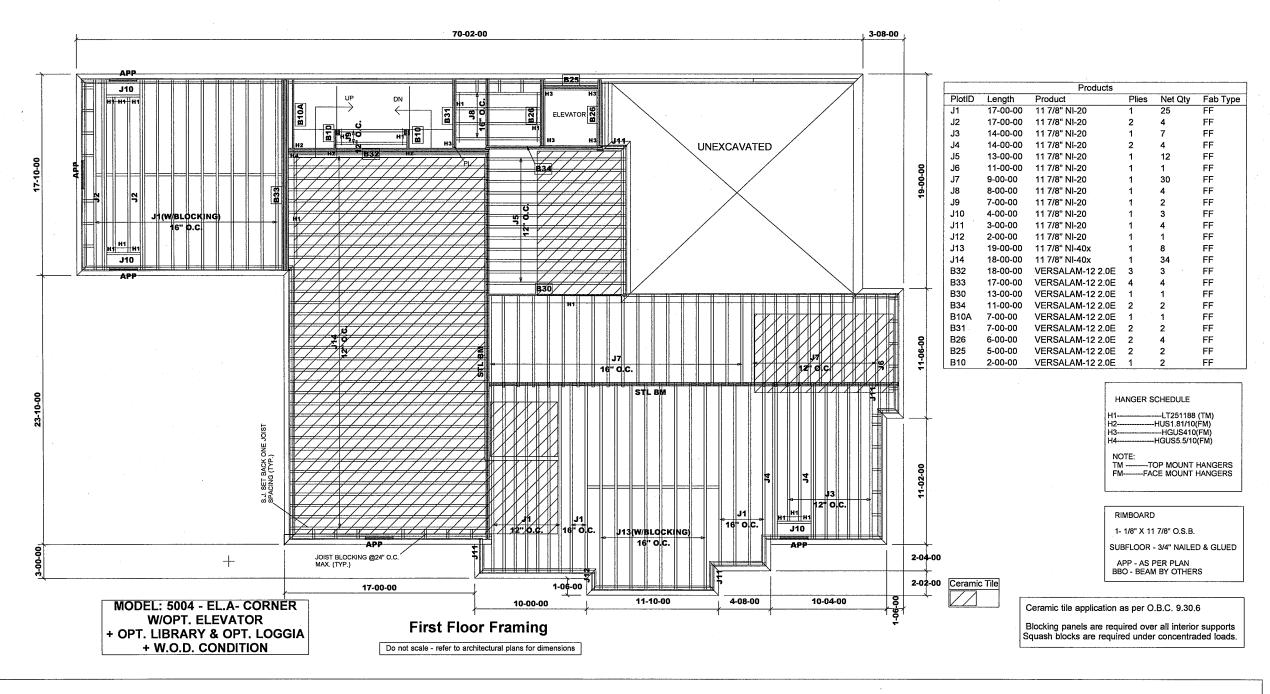
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Designer: NL

Sheet: 6 of 10

Alpa Roof Trusses Inc.
Maple, Ontario

Salesperson: Derek



LI: 318278(290673)

Builder: Gold Park

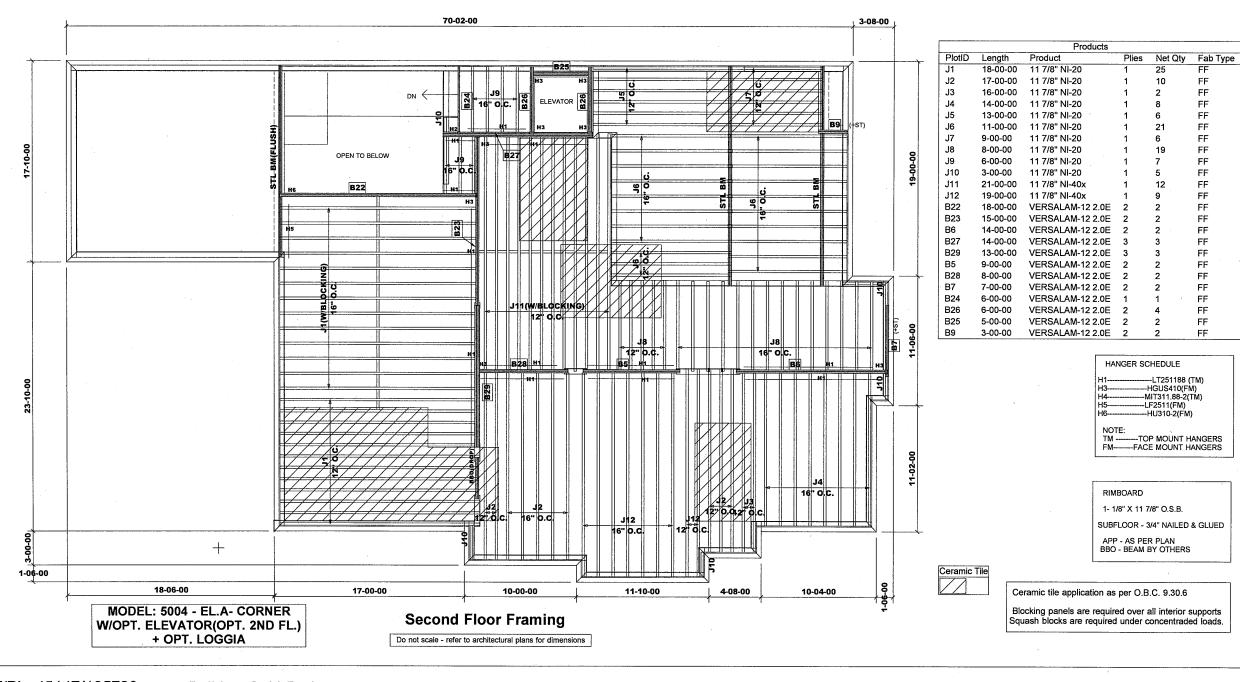
Project: Pine Valley

Location: Vaughan

Date: March 20, 2020

Designer: NL Sheet: 7 of 10 Alpa Roof Trusses Inc.
Maple, Ontario

Salesperson: Derek



LI: 318278(290673)

Builder: Gold Park

Project: Pine Valley

Location: Vaughan

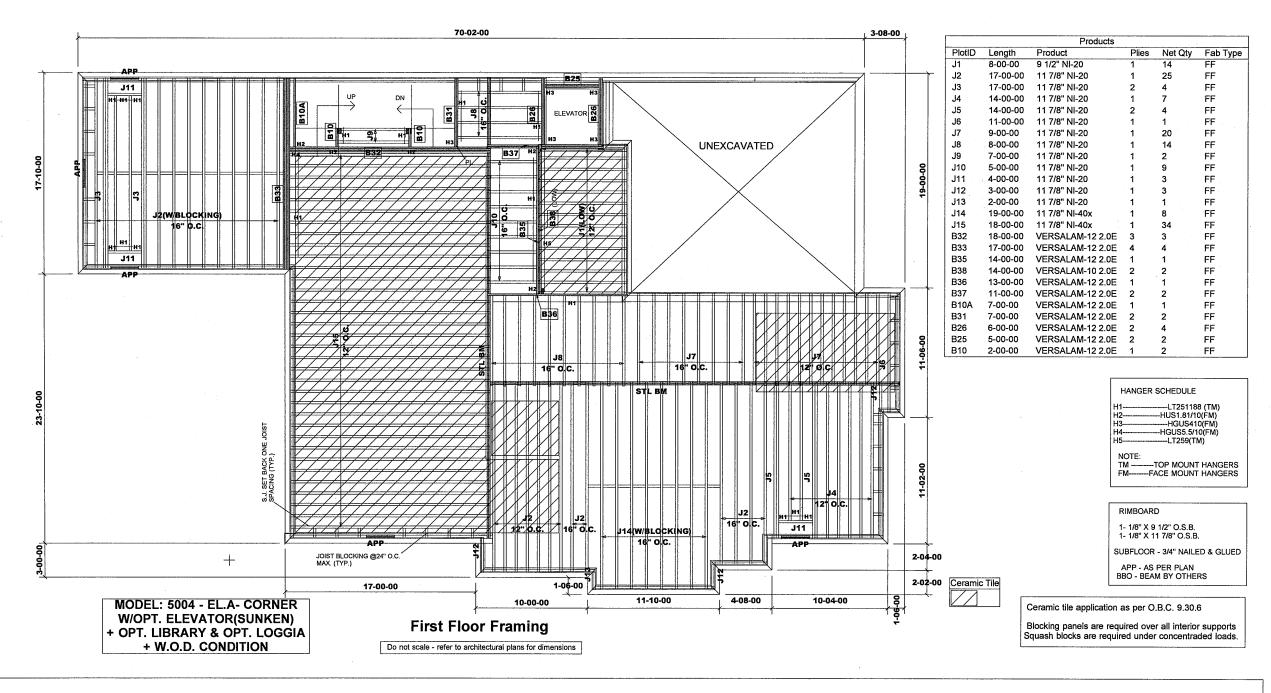
Date: March 20, 2020

Designer: NL

Sheet: 8 of 10

Alpa Roof Trusses Inc.
Maple, Ontario

Salesperson: Derek



LI: 318278(290673)

Builder: Gold Park

Project: Pine Valley

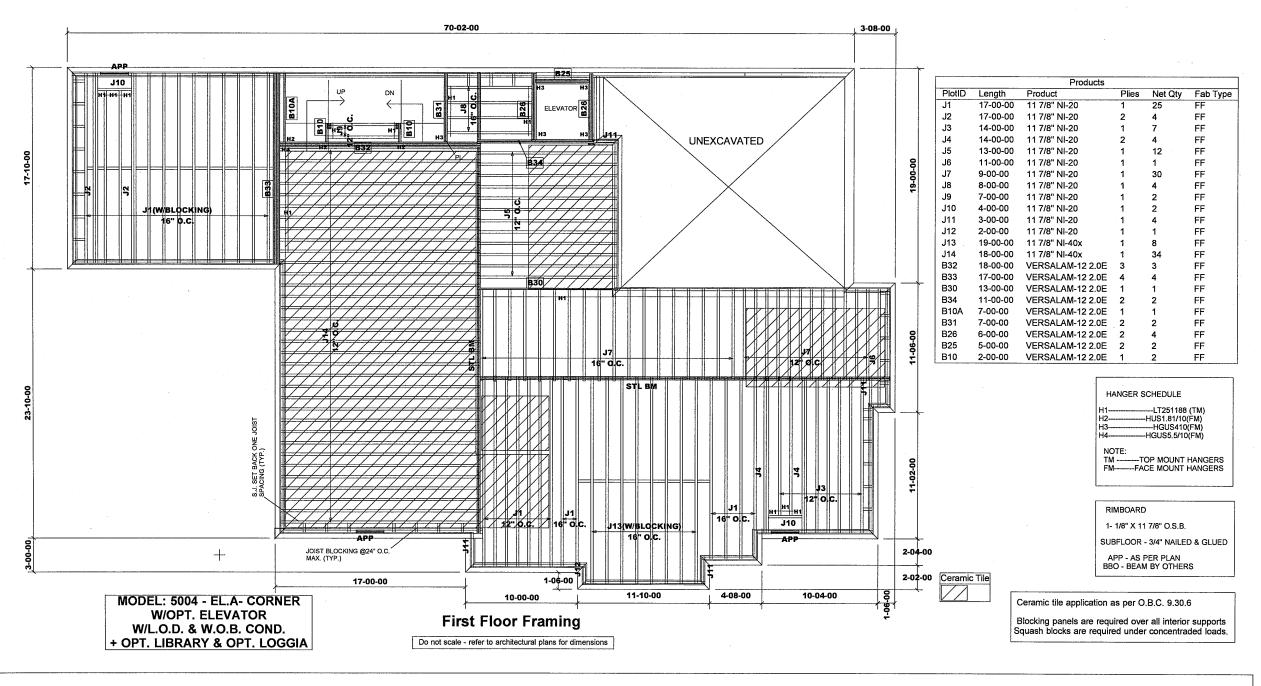
Location: Vaughan

Date: March 20, 2020

Designer: NL

Sheet: 9 of 10

Alpa Roof Trusses Inc. Maple, Ontario Salesperson: Derek



LI: 318278(290673)

Builder: Gold Park

Project: Pine Valley

Location: Vaughan

Date: March 20, 2020

Designer: NL

Sheet: 10 of 10

Alpa Roof Trusses Inc.

Maple, Ontario

Salesperson: Derek



BC CALC® Member Report

Single 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

B01 (Floor Beam)

Dry | 1 span | No cant.

Specifier:

March 20, 2020 13:19:03

PASSED

Build 7555

45147 (5004) Job name:

File name: 318278 Pine Valley Address: Description: Second Floor Framing

City, Province, Postal Code: Vaughan, ON Builder:

Gold Park Designer: NL

CCMC 12472-R Company: Code reports: Alpa Roof Trusses

10-06-00 B₀ **B1**

Total Horizontal Product Length = 10-06-00

Reaction Summary (Down / Uplift) (Ibs)

Bearing	Live	` Dead	Snow	Wind
B0, 3-1/2"	513 / 0	261 / 0		
B1, 3-1/2"	1192 / 0	505 / 0		

Lo	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	10-06-00	Тор		6			00-00-00
1		Unf. Lin. (lb/ft)	L	00-00-00	10-06-00	Top	27	14			n\a
2		Unf. Lin. (lb/ft)	L	00-00-00	06-00-00	Top	27	14			n\a
3		Unf. Area (lb/ft²)	L	06-00-00	10-06-00	Тор	40	15			07-00-00

		Factored	Demand/		
Controls Summary	Factored Demand	Resistance	Resistance	Case	Location
Pos. Moment	4209 ft-lbs	17696 ft-lbs	23.8%	1	06-07-04
End Shear	1630 lbs	7232 I bs	22.5%	1	09-02-10
Total Load Deflection	L/999 (0.103")	n\a	n\a	4	05-06-11
Live Load Deflection	L/999 (0.071")	n\a	n\a	5	05-06-11
Max Defl.	0.103"	n\a	n\a	4	05-06-11
Span / Depth	10.1				

Bearing	Supports	Dim. (LxW)	Demand	Resistance Support	Resistance Member	Material
B0	Wall/Plate	3-1/2" x 1-3/4"	1096 lbs	29.1%	14.7%	Spruce-Pine-Fir
B1	Wall/Plate	3-1/2" x 1-3/4"	2420 l bs	64.2%	32.4%	Spruce-Pine-Fir

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4



Disclosure

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

BC CALC®, BC FRAMER®, AJS™ ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,



B02 (Floor Beam)

File name:

Specifier:

318278



BC CALC® Member Report

Dry | 1 span | No cant.

March 20, 2020 13:19:03

Build 7555

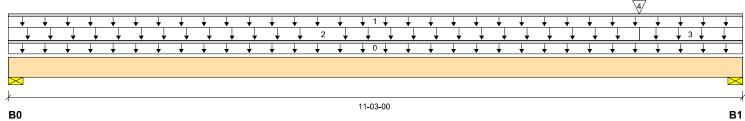
Job name: 45147 (5004)

Address: Pine Valley Description: Second Floor Framing

City, Province, Postal Code: Vaughan, ON Builder: Gold Park

Gold Park Designer: NL

Code reports: CCMC 12472-R Company: Alpa Roof Trusses



Total Horizontal Product Length = 11-03-00

Reaction Summary (Down / Uplift) (lbs)

Reaction our	ililiai y (Dowii / Op				
Bearing	Live	Dead	Snow	Wind	
B0, 3"	2143 / 0	1297 / 0			
B1, 3-1/2"	1735 / 0	1552 / 0			

Load Summary						Live	Dead	Snow	Wind	Tributary
Tag Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0 Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	11-03-00	Тор		12			00-00-00
1	Unf. Lin. (lb/ft)	L	00-00-00	11-03-00	Top	27	74			n∖a
2	Unf. Area (lb/ft²)	L	00-00-00	09-08-00	Тор	40	15			09-00-00
3	Unf. Area (lb/ft²)	L	09-08-00	11-03-00	Тор	40	15			01-06-00
4	Conc. Pt. (lbs)	L	09-08-00	09-08-00	Top		540			n\a

		Factored	Demand/		
Controls Summary	Factored Demand	Resistance	Resistance	Case	Location
Pos. Moment	12755 ft-lbs	35392 ft-lbs	36.0%	1	05-07-00
End Shear	4201 lbs	14464 I bs	29.0%	1	09-11-10
Total Load Deflection	L/659 (0.197")	n\a	36.4%	4	05-07-00
Live Load Deflection	L/999 (0.119")	n\a	n\a	5	05-07-00
Max Defl.	0.197"	n\a	19.7%	4	05-07-00
Span / Depth	10.9				

Bearin	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	3" x 3-1/2"	4836 I bs	74.9%	37.8%	Spruce-Pine-Fir
B1	Wall/Plate	3-1/2" x 3-1/2"	4542 I bs	60.3%	30.4%	Spruce-Pine-Fir



Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4

NAIL ONE PLY TO ANOTHER WITH 3-1/2" SPIRAL NAILS @ 9" O/C, STAGGERED IN 2 ROWS



B03 (Floor Beam)

Specifier:

PASSED

BC CALC® Member Report

Dry | 1 span | No cant.

March 20, 2020 13:19:03

Build 7555

B1, 3-1/2"

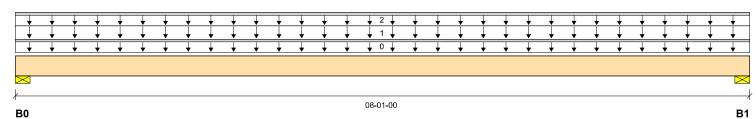
45147 (5004) Job name:

318278 File name: Pine Valley Address: Description: Second Floor Framing

City, Province, Postal Code: Vaughan, ON Builder:

Gold Park Designer: NL

Code reports: CCMC 12472-R Company: Alpa Roof Trusses



Total Horizontal Product Length = 08-01-00

Reaction Summary (Down / Uplift) (lbs)

2506 / 0

Bearing Live Dead Snow Wind B0, 3-1/2" 1544 / 0 2506 / 0

1544 / 0

	Loa	d Summary						Live	Dead	Snow	Wind	Tributary
_	Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
	0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	08-01-00	Тор		12			00-00-00
	1		Unf. Area (lb/ft²)	L	00-00-00	08-01-00	Top	40	20			15-06-00
	2		Unf. Lin. (lb/ft)	L	00-00-00	08-01-00	Top		60			n∖a

		Factored	Demand/		
Controls Summary	Factored Demand	Resistance	Resistance	Case	Location
Pos. Moment	10229 ft-lbs	35392 ft-lbs	28.9%	1	04-00-08
End Shear	3885 lbs	14464 I bs	26.9%	1	01-03-06
Total Load Deflection	L/999 (0.078")	n\a	n\a	4	04-00-08
Live Load Deflection	L/999 (0.048")	n\a	n\a	5	04-00-08
Max Defl.	0.078"	n\a	n∖a	4	04-00-08
Snan / Denth	7 7				

Bearing	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	3-1/2" x 3-1/2"	5689 lbs	75.5%	38.1%	Spruce-Pine-Fir
B1	Wall/Plate	3-1/2" x 3-1/2"	5689 lbs	75.5%	38.1%	Spruce-Pine-Fir



Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4

NAIL ONE PLY TO ANOTHER WITH 3-1/2" SPIRAL NAILS @ 9" O/C, STAGGERED IN 2 ROWS



B04 (Floor Beam)

PASSED

March 20, 2020 13:19:03

BC CALC® Member Report

Dry | 1 span | No cant.

Build 7555

45147 (5004) Job name:

Pine Valley Address: Description: Second Floor Framing

City, Province, Postal Code: Vaughan, ON Builder:

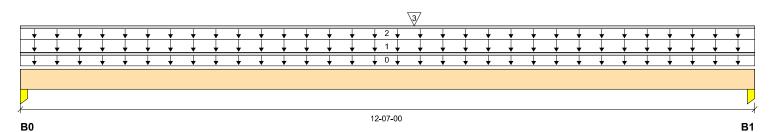
Gold Park

Code reports: CCMC 12472-R File name: 318278

Specifier:

Designer:

NL Company: Alpa Roof Trusses



Total Horizontal Product Length = 12-07-00

Reaction Summary (Down / Uplift) (lbs)

Bearing Live Dead Snow Wind B0, 3-1/2" 3561 / 0 2370 / 0 B1, 3" 3726 / 0 2471 / 0

Lo	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag		Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	12-07-00	Тор		12			00-00-00
1		Unf. Area (Ib/ft²)	L	00-00-00	12-07-00	Top	40	20			09-06-00
2		Unf. Lin. (lb/ft)	L	00-00-00	12-07-00	Тор		60			n\a
3		Conc. Pt. (lbs)	L	06-09-00	06-09-00	Top	2506	1544			n∖a

		Factored	Demand/		
Controls Summary	Factored Demand	Resistance	Resistance	Case	Location
Pos. Moment	33735 ft-lbs	35392 ft-lbs	95.3%	1	06-09-00
End Shear	7566 lbs	14464 I bs	52.3%	1	11-04-02
Total Load Deflection	L/247 (0.591")	n∖a	97.1%	4	06-04-14
Live Load Deflection	L/409 (0.357")	n\a	88.0%	5	06-04-14
Max Defl.	0.591"	n\a	59.1%	4	06-04-14
Span / Depth	12.3				

Bearing	Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Column	3-1/2" x 3-1/2"	8305 lbs	39.1%	55.6%	Spruce-Pine-Fir
B1	Column	3" x 3-1/2"	8678 lbs	47.6%	67.7%	Spruce-Pine-Fir



Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4

NAIL ONE PLY TO ANOTHER WITH 3-1/2" SPIRAL NAILS @ 9" O/C, STAGGERED IN 2 ROWS



B05 (Floor Beam)

File name:

Specifier:

318278

PASSED

March 20, 2020 13:19:03

BC CALC® Member Report

Dry | 1 span | No cant.

Build 7555

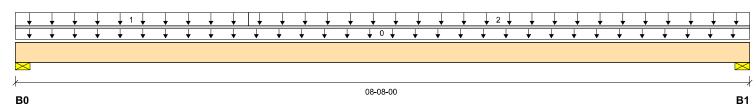
45147 (5004) Job name:

Pine Valley Address: Description: Second Floor Framing

City, Province, Postal Code: Vaughan, ON Builder:

Gold Park Designer: NL

Code reports: CCMC 12472-R Company: Alpa Roof Trusses



Total Horizontal Product Length = 08-08-00

Reaction Summary (Down / Uplift) (lbs)

Live Dead Snow B0, 3-1/2" 2679 / 0 1391 / 0 B1, 3-1/2" 2323 / 0 1214 / 0

Lo	Load Summary						Live	Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	08-08-00	Тор		12			00-00-00
1		Unf. Area (lb/ft²)	L	00-00-00	02-09-00	Top	40	20			17-06-00
2		Unf. Area (Ib/ft²)	L	02-09-00	08-08-00	Тор	40	20			13-00-00

		Factored	Demand/		
Controls Summary	Factored Demand	Resistance	Resistance	Case	Location
Pos. Moment	10050 ft-lbs	35392 ft-lbs	28.4%	1	04-02-06
End Shear	3832 lbs	14464 I bs	26.5%	1	01-03-06
Total Load Deflection	L/999 (0.089")	n\a	n\a	4	04-03-04
Live Load Deflection	L/999 (0.058")	n\a	n\a	5	04-03-04
Max Defl.	0.089"	n\a	n\a	4	04-03-04
Span / Depth	8.3				

Bearing	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	3-1/2" x 3-1/2"	5757 lbs	76.4%	38.5%	Spruce-Pine-Fir
B1	Wall/Plate	3-1/2" x 3-1/2"	5001 lbs	66.4%	33.5%	Spruce-Pine-Fir



Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4

NAIL ONE PLY TO ANOTHER WITH 3-1/2" SPIRAL NAILS @ STAGGERED IN 2 ROWS



B06 (Floor Beam)

File name:

Specifier:

318278

PASSED

BC CALC® Member Report

Dry | 1 span | No cant.

March 20, 2020 13:19:03

Build 7555

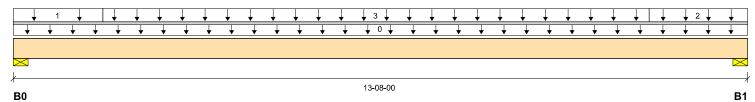
Job name: 45147 (5004)

Pine Valley Description: Second Floor Framing Address:

City, Province, Postal Code: Vaughan, ON

Builder: Gold Park Designer: NL

Code reports: CCMC 12472-R Company: Alpa Roof Trusses



Total Horizontal Product Length = 13-08-00

Reaction Sur	ililiary (Down / Op	onit) (105)			
Bearing	Live	Dead	Snow	Wind	
B0, 3-1/2"	3049 / 0	1604 / 0			
B1, 3-1/2"	2628 / 0	1348 / 0			

Lo	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	13-08-00	Тор		12			00-00-00
1		Unf. Area (Ib/ft²)	L	00-00-00	01-08-00	Top	40	20			12-00-00
2		Unf. Area (Ib/ft²)	L	11-10-00	13-08-00	Top	40	15			05-06-00
3		Unf. Area (lb/ft²)	L	01-08-00	11-10-00	Тор	40	20			11-00-00

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	20441 ft-lbs	35392 ft-lbs	57.8%	1	06-09-00
End Shear	5252 l bs	14464 I bs	36.3%	1	01-03-06
Total Load Deflection	L/342 (0.463")	n\a	70.2%	4	06-09-00
Live Load Deflection	L/522 (0.304")	n\a	69.0%	5	06-09-00
Max Defl.	0.463"	n\a	46.3%	4	06-09-00
Span / Depth	13.3				

Bearin	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	3-1/2" x 3-1/2"	6578 I bs	87.3%	44.0%	Spruce-Pine-Fir
B1	Wall/Plate	3-1/2" x 3-1/2"	5627 lbs	74.7%	37.7%	Spruce-Pine-Fir



Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4

NAIL ONE PLY TO ANOTHER WITH 3-1/2" SPIRAL NAILS @ 10" O/C, STAGGERED IN 2 ROWS



B07 (Floor Beam)

Specifier:

Designer:

NL

March 20, 2020 13:19:03 Dry | 1 span | No cant.

BC CALC® Member Report

Build 7555

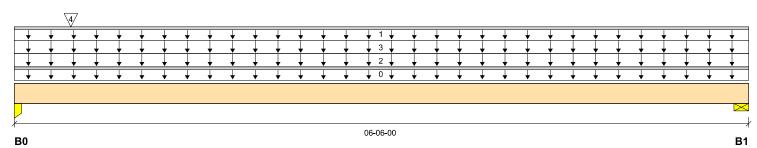
45147 (5004) Job name:

318278 File name: Pine Valley Address: Description: Second Floor Framing

City, Province, Postal Code: Vaughan, ON

Builder: Gold Park

Code reports: CCMC 12472-R Company: Alpa Roof Trusses



Total Horizontal Product Length = 06-06-00

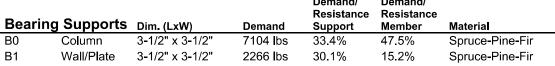
Reaction Summary (Down / Uplift) (Ibs)

Bearing	Live	Dead	Snow	Wind	
B0, 3-1/2"	2597 / 0	1950 / 0	770 / 0		
B1 3-1/2"	206 / 0	724 / 0	770 / 0		

Load Summa	ry					Live	Dead	Snow	Wind	Tributary
Tag Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0 Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	06-06-00	Тор		12			00-00-00
1	Unf. Lin. (lb/ft)	L	00-00-00	06-06-00	Top	27	114			n∖a
2	Unf. Area (Ib/ft²)	L	00-00-00	06-06-00	Тор		20	78		02-06-00
3	Unf. Area (lb/ft²)	L	00-00-00	06-06-00	Тор		14	21		02-00-00
4	Conc. Pt. (lbs)	L	00-06-00	00-06-00	Top	2628	1348			n∖a

		Factored	Demand/		
Controls Summary	Factored Demand	Resistance	Resistance	Case	Location
Pos. Moment	3526 ft-lbs	35392 ft-lbs	10.0%	5	02-11-05
End Shear	1979 lbs	14464 I bs	13.7%	1	01-03-06
Total Load Deflection	L/999 (0.018")	n\a	n\a	11	03-01-08
Live Load Deflection	L/999 (0.01")	n\a	n\a	15	03-01-08
Max Defl.	0.018"	n\a	n\a	11	03-01-08
Span / Depth	6.1				

Bear	ing Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
В0	Column	3-1/2" x 3-1/2"	7104 lbs	33.4%	47.5%	Spruce-Pine-Fir
B1	Wall/Plate	3-1/2" x 3-1/2"	2266 lbs	30.1%	15.2%	Spruce-Pine-Fir





Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4

NAIL ONE PLY TO ANOTHER WITH 3-1/2" SPIRAL NAILS @ 12" O/C, STAGGERED IN 2 ROWS



PASSED



B08 (Floor Beam)

Specifier:

Designer:

318278

NL

Dry | 1 span | No cant.

PASSED

March 20, 2020 13:19:03

BC CALC® Member Report

Build 7555

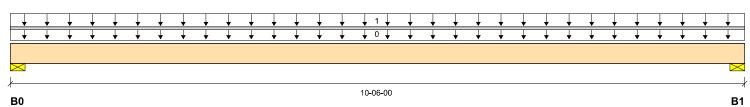
45147 (5004) Job name:

File name: Pine Valley Address: Description: Second Floor Framing

City, Province, Postal Code: Vaughan, ON

Builder: Gold Park

Alpa Roof Trusses CCMC 12472-R Company: Code reports:



Total Horizontal Product Length = 10-06-00

Reaction Summary (Down / Uplift) (lbs)

Snow Live Dead B0, 3-1/2" 1260 / 0 662 / 0 B1, 3-1/2" 1260 / 0 662 / 0

Loa	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	10-06-00	Тор		6			00-00-00
1		Unf. Area (lb/ft²)	L	00-00-00	10-06-00	Top	40	20			06-00-00

		Factored	Demand/		
Controls Summary	Factored Demand	Resistance	Resistance	Case	Location
Pos. Moment	6523 ft-lbs	17696 ft-lbs	36.9%	1	05-03-00
End Shear	2054 lbs	7232 lbs	28.4%	1	01-03-06
Total Load Deflection	L/703 (0.171")	n\a	34.1%	4	05-03-00
Live Load Deflection	L/999 (0.112")	n\a	n\a	5	05-03-00
Max Defl.	0.171"	n\a	17.1%	4	05-03-00
Span / Depth	10.1				

Bearing	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	3-1/2" x 1-3/4"	2717 lbs	72.1%	36.4%	Spruce-Pine-Fir
B1	Wall/Plate	3-1/2" x 1-3/4"	2717 I bs	72.1%	36.4%	Spruce-Pine-Fir



Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4

Disclosure

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

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B09 (Floor Beam)

File name:

Specifier:

318278

PASSED

BC CALC® Member Report

Dry | 1 span | No cant.

March 20, 2020 13:19:03

Build 7555

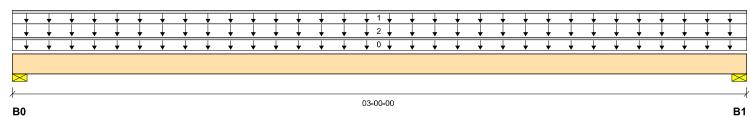
45147 (5004) Job name:

Pine Valley Address: Description: Second Floor Framing

City, Province, Postal Code: Vaughan, ON Builder:

Gold Park Designer: NL

Code reports: CCMC 12472-R Company: Alpa Roof Trusses



Total Horizontal Product Length = 03-00-00

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B0, 3-1/2"	40 / 0	378 / 0	284 / 0	
B1. 3-1/2"	40 / 0	378 / 0	283 / 0	

Loa	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	03-00-00	Тор		12			00-00-00
1		Unf. Lin. (lb/ft)	L	00-00-00	03-00-00	Тор	27	114			n∖a
2		Unf. Area (lb/ft²)	L	00-00-00	03-00-00	Top		14	21		09-00-00

		Factored	Demand/		
Controls Summary	Factored Demand	Resistance	Resistance	Case	Location
Pos. Moment	505 ft-lbs	35392 ft-lbs	1.4%	5	01-06-00
End Shear	137 lbs	14464 I bs	0.9%	5	01-03-06
Total Load Deflection	L/999 (0")	n\a	n\a	11	01-06-00
Live Load Deflection	L/999 (0")	n\a	n\a	15	01-06-00
Max Defl.	0"	n\a	n\a	11	01-06-00
Span / Depth	2.6				

Bearing	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	3-1/2" x 3-1/2"	938 lbs	12.5%	6.3%	Spruce-Pine-Fir
B1	Wall/Plate	3-1/2" x 3-1/2"	938 lbs	12.5%	6.3%	Spruce-Pine-Fir



Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4

NAIL ONE PLY TO ANOTHER WITH 3-1/2" SPIRAL NAILS @ 4" O/C, STAGGERED IN 2 ROWS



B10 (Floor Beam)

Dry | 1 span | No cant.

PASSED

March 20, 2020 13:19:03

BC CALC® Member Report

Build 7555

Code reports:

45147 (5004) Job name:

Pine Valley Address: Description:

City, Province, Postal Code: Vaughan, ON Builder:

Gold Park

CCMC 12472-R

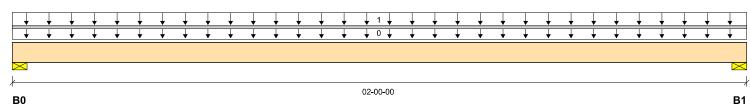
File name: 318278

First Floor Framing

Specifier:

Designer: NL

Alpa Roof Trusses Company:



Total Horizontal Product Length = 02-00-00

Snow

Reaction Summary (Down / Uplift) (lbs)

Live Dead B0, 3-1/2" 140 / 0 59 / 0 B1, 3-1/2" 140 / 0 59 / 0

	ad Summary Description	Load Type	Ref.	Start	End	Loc.	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Tributary
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	02-00-00	Тор		6			00-00-00
1		Unf. Area (Ib/ft²)	L	00-00-00	02-00-00	Тор	40	15			03-06-00

		Factored	Demand/		
Controls Summary	Factored Demand	Resistance	Resistance	Case	Location
Pos. Moment	84 ft-lbs	17696 ft-lbs	0.5%	1	01-00-00
End Shear	80 l bs	7232 I bs	1.1%	1	01-03-06
Total Load Deflection	L/999 (0")	n\a	n\a	4	01-00-00
Max Defl.	0"	n\a	n\a	4	01-00-00
Span / Depth	1.6				

Bea	ring Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Materia l
B0	Wall/Plate	3-1/2" x 1-3/4"	283 lbs	7.5%	3.8%	Spruce-Pine-Fir
B1	Wall/P l ate	3-1/2" x 1-3/4"	283 lbs	7.5%	3.8%	Spruce-Pine-Fir



Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4

Disclosure

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BC CALC® Member Report

Single 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

B10A (Floor Beam)

File name:

Specifier:

318278

Dry | 1 span | No cant.

March 20, 2020 13:19:03

PASSED

Build 7555

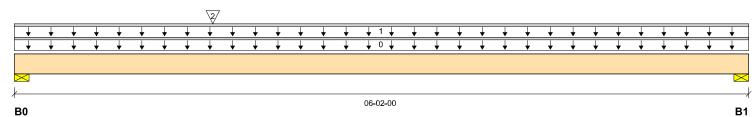
45147 (5004) Job name:

Pine Valley Address: Description: First Floor Framing

City, Province, Postal Code: Vaughan, ON

Gold Park Designer: NL

Builder: Alpa Roof Trusses CCMC 12472-R Company: Code reports:



Total Horizontal Product Length = 06-02-00

Reaction Summary (Down / Uplift) (Ibs)

Bearing	Live	. ´ `Dead	Snow	Wind	
B0, 3-1/2"	331 / 0	335 / 0			
B1, 3-1/2"	152 / 0	268 / 0			

Loa	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	06-02-00	Тор		6			00-00-00
1		Unf. Lin. (lb/ft)	L	00-00-00	06-02-00	Top	20	70			n\a
2		Conc. Pt. (lbs)	L	01-08-00	01-08-00	Top	360	135			n∖a

		Factored	Demand/		
Controls Summary	Factored Demand	Resistance	Resistance	Case	Location
Pos. Moment	1146 ft-lbs	17696 ft-lbs	6.5%	1	01-08-00
End Shear	756 lbs	7232 lbs	10.4%	1	01-03-06
Total Load Deflection	L/999 (0.009")	n\a	n\a	4	02-10-08
Live Load Deflection	L/999 (0.004")	n\a	n\a	5	02-09-04
Max Defl.	0.009"	n\a	n\a	4	02-10-08
Span / Depth	5.8				

Bearin	ng Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	3-1/2" x 1-3/4"	916 I bs	24.3%	12.3%	Spruce-Pine-Fir
B1	Wall/Plate	3-1/2" x 1-3/4"	376 I bs	15.3%	7.7%	Spruce-Pine-Fir

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4



Disclosure

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BC CALC®, BC FRAMER®, AJS™ ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,



0 3100 SP PASSED

March 20, 2020 13:19:03

B11 (Floor Beam)

File name:

Specifier:

318278

NL

BC CALC® Member Report

Dry | 1 span | No cant.

Build 7555

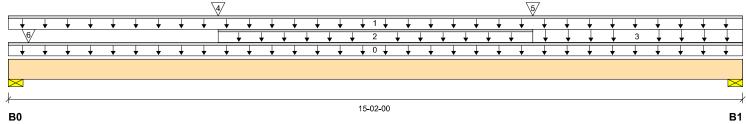
Job name: 45147 (5004)

Address: Pine Valley Description: First Floor Framing

City, Province, Postal Code: Vaughan, ON

Builder: Gold Park Designer:

Code reports: CCMC 12472-R Company: Alpa Roof Trusses



Total Horizontal Product Length = 15-02-00

Reaction Summary (Down / Uplift) (lbs)

i toaotioii oai	a.	p()			
Bearing	Live	Dead	Snow	Wind	
B0, 3-1/2"	782 / 0	596 / 0			
B1, 3-1/2"	587 / 0	317 / 0			

Load Summary						Live	Dead	Snow	Wind	Tributary
Tag Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0 Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	15-02-00	Тор		6			00-00-00
1	Unf. Lin. (lb/ft)	L	00-00-00	15-02-00	Top	27	14			n\a
2	Unf. Lin. (lb/ft)	L	04-04-00	10-10-00	Top	27	14			n\a
3	Unf. Area (lb/ft²)	L	10-10-00	15-02-00	Top	40	15			01-00-00
4	Conc. Pt. (lbs)	L	04-04-00	04-04-00	Top	140	59			n∖a
5	Conc. Pt. (lbs)	L	10-10-00	10-10-00	Top	140	59			n∖a
6	Conc. Pt. (lbs)	L	00-05-00	00-05-00	Top	331	335			n∖a

		Factored	Demand/		
Controls Summary	Factored Demand	Resistance	Resistance	Case	Location
Pos. Moment	4438 ft-lbs	17696 ft-lbs	25.1%	1	07-09-07
End Shear	1092 lbs	7232 lbs	15.1%	1	13-10-10
Total Load Deflection	L/691 (0.255")	n\a	34.7%	4	07-07-00
Live Load Deflection	L/1081 (0.163")	n\a	33.3%	5	07-07-00
Max Defl.	0.255"	n\a	25.5%	4	07-07-00
Span / Depth	14.9				

Bearing	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	3-1/2" x 1-3/4"	1918 lbs	50.9%	25.7%	Spruce-Pine-Fir
B1	Wall/Plate	3-1/2" x 1-3/4"	1277 I bs	33.9%	17.1%	Spruce-Pine-Fir



Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

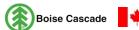
Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4



PASSED

В1

B12 (Floor Beam)

File name:

Specifier:

Designer:

318278

Alpa Roof Trusses

Wind

NL

BC CALC® Member Report Dry | 1 spa

Dry | 1 span | No cant.

March 20, 2020 13:19:03

Build 7555

B0

Job name: 45147 (5004)

Address: Pine Valley Description: First Floor Framing

City, Province, Postal Code: Vaughan, ON Builder: Gold Park

Code reports: CCMC 12472-R Company:

Total Horizontal Product Length = 17-00-00

Reaction Summary (Down / Uplift) (Ibs)

 Bearing
 Live
 Dead
 Snow

 B0, 3-1/2"
 3263 / 0
 1552 / 0

 B1, 3-1/2"
 2229 / 0
 1278 / 0

Load Summary						Live	Dead	Snow	Wind	Tributary
Tag Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0 Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	17-00-00	Тор		18			00-00-00
1	Unf. Lin. (lb/ft)	L	00-00-00	17-00-00	Top	27	14			n∖a
2	Unf. Area (Ib/ft²)	L	00-00-00	07-00-00	Тор	40	15			09-00-00
3	Unf. Area (Ib/ft²)	L	07-00-00	10-10-00	Тор	40	15			07-06-00
4	Unf. Lin. (lb/ft)	L	10-10-00	17-00-00	Тор	20	8			n∖a
5	Conc. Pt. (lbs)	L	10-10-00	10-10-00	Тор	782	596			n∖a
6	Conc. Pt. (lbs)	L	10-10-00	10-10-00	Тор	457	264			n\a

		Factored	Demand/		
Controls Summary	Factored Demand	Resistance	Resistance	Case	Location
Pos. Moment	28235 ft-lbs	55212 ft-lbs	51.1%	1	08-11-00
End Shear	5823 lbs	21696 lbs	26.8%	1	01-03-06
Total Load Deflection	L/301 (0.659")	n\a	79.7%	4	08-05-04
Live Load Deflection	L/459 (0.433")	n\a	78.4%	5	08-05-04
Max Defl.	0.659"	n\a	65.9%	4	08-05-04
Span / Depth	16.7				

Bear	ing Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	3-1/2" x 5-1/4"	6835 lbs	60.5%	30.5%	Spruce-Pine-Fir
B1	Wall/Plate	3-1/2" x 5-1/4"	4940 I bs	43.7%	22.0%	Spruce-Pine-Fir



Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4

NAIL ONE PLY TO ANOTHER WITH 3-1/2" SPIRAL NAILS @ 6" O/C, STAGGERED IN 2 ROWS



B13 (Floor Beam)

File name:

Specifier:

318278

BC CALC® Member Report

Dry | 1 span | No cant.

March 20, 2020 13:19:03

PASSED

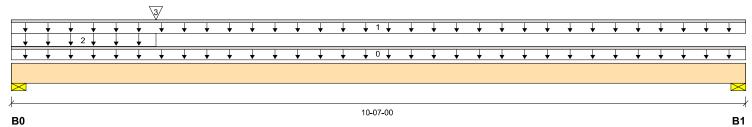
Build 7555

Job name: 45147 (5004)

Pine Valley Description: Second Floor Framing Address:

City, Province, Postal Code: Vaughan, ON Builder: Gold Park

Designer: NL Code reports: CCMC 12472-R Company: Alpa Roof Trusses



Total Horizontal Product Length = 10-07-00

Reaction Summary (Down / Uplift) (lbs)

reaction cannot y (20111) (180)						
Bearing	Live	Dead	Snow	Wind		
B0, 3-1/2"	143 / 0	1258 / 0	828 / 0			
B1, 3-1/2"	143 / 0	781 / 0	160 / 0			

Lo	ad Summary						Live	Dead	Snow	Wind	Tributary
	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	10-07-00	Тор		12			00-00-00
1		Unf. Lin. (Ib/ft)	L	00-00-00	10-07-00	Top	27	114			n\a
2		Unf. Area (Ib/ft²)	L	00-00-00	02-01-00	Тор		20	28		03-06-00
3		Conc. Pt. (lbs)	L	02-01-00	02-01-00	Top		560	784		n∖a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	3144 ft-lbs	23005 ft-lbs	13.7%	0	04-04-01
End Shear	2421 lbs	14464 I bs	16.7%	5	01-03-06
Total Load Deflection	L/999 (0.066")	n\a	n\a	11	05-00-03
Live Load Deflection	L/999 (0.023")	n\a	n\a	15	04-09-08
Max Defl.	0.066"	n\a	n\a	11	05-00-03
Span / Depth	10.2				

Bearing	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	3-1/2" x 3-1/2"	2958 lbs	39.2%	19.8%	Spruce-Pine-Fir
B1	Wall/Plate	3-1/2" x 3-1/2"	1094 I bs	22.3%	11.3%	Spruce-Pine-Fir



Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4

NAIL ONE PLY TO ANOTHER WITH 3-1/2" SPIRAL NAILS @ 12" O/C. STAGGERED IN 2 ROWS



B14 (Floor Beam)

File name:

Specifier:

318278



BC CALC® Member Report

Dry | 1 span | No cant.

March 20, 2020 13:19:03

Build 7555

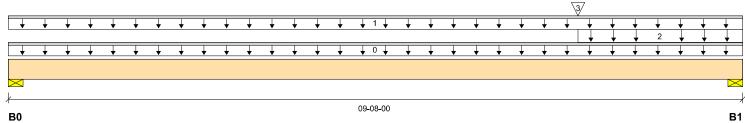
Job name: 45147 (5004)

Address: Pine Valley Description: Second Floor Framing

City, Province, Postal Code: Vaughan, ON Builder: Gold Park

Gold Park Designer: NL

Code reports: CCMC 12472-R Company: Alpa Roof Trusses



Total Horizontal Product Length = 09-08-00

Reaction Summary (Down / Uplift) (lbs)

readucti daminary (Domit, (186)									
Bearing	Live	Dead	Snow	Wind					
B0, 3-1/2"	131 / 0	741 / 0	185 / 0						
B1, 3-1/2"	130 / 0	1189 / 0	811 / 0						

Lo	ad Summary						Live	Dead	Snow	Wind	Tributary
	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	09-08-00	Тор		12			00-00-00
1		Unf. Lin. (lb/ft)	L	00-00-00	09-08-00	Top	27	114			n\a
2		Unf. Area (Ib/ft²)	L	07-06-00	09-08-00	Тор		20	28		03-06-00
3		Conc. Pt. (lbs)	L	07-06-00	07-06-00	Тор		560	784		n∖a

		Factored	Demand/		
Controls Summary	Factored Demand	Resistance	Resistance	Case	Location
Pos. Moment	4525 ft-lbs	35392 ft-lbs	12.8%	5	07-02-08
End Shear	2297 lbs	14464 I bs	15.9%	5	08-04-10
Total Load Deflection	L/999 (0.05")	n\a	n\a	11	05-02-00
Live Load Deflection	L/999 (0.018")	n\a	n\a	15	05-03-03
Max Defl.	0.05"	n\a	n\a	11	05-02-00
Span / Depth	9.3				

Bear	ing Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	3-1/2" x 3-1/2"	1038 lbs	21.2%	10.7%	Spruce-Pine-Fir
B1	Wall/Plate	3-1/2" x 3-1/2"	2834 lbs	37.6%	19.0%	Spruce-Pine-Fir



Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4

NAIL ONE PLY TO ANOTHER WITH 3-1/2" SPIRAL NAILS @ 12" O/C,

STAGGERED IN 2 ROWS



PASSED

B15 (Floor Beam)

File name:

Specifier:

Company:

318278

Alpa Roof Trusses

BC CALC® Member Report

Dry | 2 spans | R cant.

March 20, 2020 13:19:03

Build 7555

Code reports:

Job name: 45147 (5004)

Address: Pine Valley Description: Second Floor Framing

City, Province, Postal Code: Vaughan, ON Builder: Gold Park

Gold Park Designer: NL

B0

13-11-00

B1

Total Horizontal Product Length = 15-07-00

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B0, 3-1/2"	3057 / 56	1282 / 0	0 / 104	
B1. 3-1/2"	3708 / 0	3244 / 0	978 / 0	

CCMC 12472-R

Load Summary						Live	Dead	Snow	Wind	Tributary
Tag Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0 Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	15-07-00	Тор		18			00-00-00
1	Unf. Area (lb/ft²)	L	00-00-00	15-07-00	Тор	40	15			05-04-00
2	Unf. Area (lb/ft²)	L	00-00-00	11-10-00	Тор	40	20			05-06-00
3	Unf. Lin. (lb/ft)	L	13-11-00	15-07-00	Тор		120	28		n∖a
4	Conc. Pt. (lbs)	L	15-07-00	15-07-00	Тор	143	1258	828		n∖a
5	Unf. Area (lb/ft²)	L	11-10-00	15-07-00	Тор	40	15			04-03-00

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	19644 ft-lbs	55212 ft-lbs	35.6%	8	06-08-11
Neg. Moment	-6009 ft-lbs	-55212 ft-lbs	10.9%	15	13-11-00
End Shear	5023 l bs	21696 lbs	23.2%	8	01-03-06
Cont. Shear	5505 l bs	21696 I bs	25.4%	1	12-09-06
Total Load Deflection	L/532 (0.309")	n\a	45.1%	23	06-10-10
Live Load Deflection	L/708 (0.232")	n\a	50.9%	33	07-00-10
Total Neg. Defl.	2xL/1998 (-0.108")	n\a	n\a	23	15-07-00
Max Defl.	0.309"	n\a	30.9%	23	06-10-10
Cant. Max Defl.	-0.108"	n\a	n\a	23	15-07-00
Span / Depth	13.8				



Bearing	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	3-1/2" x 5-1/4"	6189 lbs	54.7%	27.6%	Spruce-Pine-Fir
B1	Wall/Plate	3-1/2" x 5-1/4"	10595 lbs	93.7%	47.3%	Spruce-Pine-Fir

NAIL ONE PLY TO ANOTHER WITH 3-1/2" SPIRAL NAILS @ 10" O/C, STAGGERED IN 2 ROWS



318278



BC CALC® Member Report

B16 (Floor Beam)

Specifier:

Dry | 3 spans | R cant.

March 20, 2020 13:19:03

Build 7555

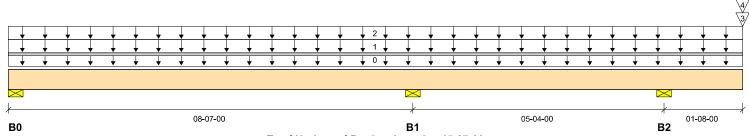
Job name: 45147 (5004)

File name: Pine Valley Description: Second Floor Framing Address:

City, Province, Postal Code: Vaughan, ON Builder:

Gold Park Designer: NL

CCMC 12472-R Code reports: Company: Alpa Roof Trusses



Total Horizontal Product Length = 15-07-00

Reaction Summary (Down / Uplift) (lbs)

	7 (1-				
Bearing	Live	Dead	Snow	Wind	
B0, 3-1/2"	1582 / 69	666 / 0	13 / 0		
B1, 3-1/2"	3681 / 0	803 / 0	0 / 142		
B2, 3-1/2"	2187 / 0	2656 / 0	474 / 0		

Load Summary						Live	Dead	Snow	Wind	Tributary
Tag Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0 Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	15-07-00	Тор		12			00-00-00
1	Unf. Area (Ib/ft²)	L	00-00-00	15-07-00	Top	40	15			05-04-00
2	Unf. Area (lb/ft²)	L	00-00-00	15-07-00	Тор	40	15			05-00-00
3	Conc. Pt. (lbs)	L	15-07-00	15-07-00	Тор	143	781	160		n∖a
4	Conc. Pt. (lbs)	L	15-07-00	15-07-00	Тор	130	741	185		n∖a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	5536 ft-lbs	35392 ft-lbs	15.6%	2	03-10-10
Neg. Moment	-3876 ft-lbs	-23005 ft-lbs	16.8%	0	13-11-00
End Shear	2157 lbs	14464 I bs	14.9%	2	01-03-06
Cont. Shear	2255 lbs	9401 I bs	24.0%	0	15-00-10
Total Load Deflection	L/999 (0.046")	n\a	n∖a	40	04-02-01
Live Load Deflection	L/999 (0.032")	n\a	n\a	56	04-02-01
Total Neg. Defl.	L/999 (-0.018")	n\a	n∖a	40	11-05-05
Max Defl.	0.046"	n\a	n∖a	40	04-02-01
Cant. Max Defl.	0.033"	n\a	n∖a	40	15-07-00
Span / Depth	8.4				



Beari	ng Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	3-1/2" x 3-1/2"	3219 lbs	42.7%	21.5%	Spruce-Pine-Fir
B1	Wall/Plate	3-1/2" x 3-1/2"	6525 l bs	86.6%	43.7%	Spruce-Pine-Fir
B2	Wall/Plate	3-1/2" x 3-1/2"	7075 lbs	93.9%	47.3%	Spruce-Pine-Fir

NAIL ONE PLY TO ANOTHER WITH 3-1/2" SPIRAL NAILS @ 12" O/C, STAGGERED IN 2 ROWS



BC CALC® Member Report



Quadruple 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

File name:

Specifier:

318278

B17 (Floor Beam)

Dry | 2 spans | R cant.

March 20, 2020 13:19:03

PASSED

Build 7555

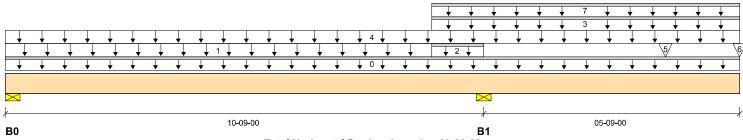
Job name: 45147 (5004)

Pine Valley Address: Description: Second Floor Framing

City, Province, Postal Code: Vaughan, ON

Builder: Gold Park Designer: NL

Code reports: CCMC 12472-R Company: Alpa Roof Trusses



Total Horizontal Product Length = 16-06-00

Reaction Summary (Down / Uplift) (Ibs)

Bearing	Live	Dead	Snow	Wind	
B0, 3-1/2"	2001 / 360	0 / 74	0 / 486		
B1. 5-1/2"	3272 / 0	4789 / 0	1513 / 0		

Load Summary						Live	Dead	Snow	Wind	Tributary
Tag Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0 Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	16-06-00	Тор		24			00-00-00
1	Unf. Area (lb/ft²)	L	00-00-00	09-07-00	Тор	40	20			04-07-00
2	Unf. Lin. (Ib/ft)	L	09-07-00	10-09-00	Тор		14	21		n∖a
3	Unf. Lin. (lb/ft)	L	09-07-00	16-06-00	Top		100			n∖a
4	Unf. Area (lb/ft²)	L	00-00-00	16-06-00	Тор	40	15			04-07-00
5	Conc. Pt. (lbs)	L	14-10-00	14-10-00	Тор		270			n\a
6	Conc. Pt. (lbs)	L	16-06-00	16-06-00	Top	131	1189	811		n\a
7	Unf. Lin. (lb/ft)	L	09-07-00	16-06-00	Тор		20	28		n∖a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	5362 ft-lbs	73615 ft-lbs	7.3%	23	04-01-07
Neg. Moment	-25795 ft-lbs	-73615 ft-lbs	35.0%	28	10-09-00
End Shear	2018 lbs	28927 lbs	7.0%	23	01-03-06
Cont. Shear	5777 I bs	28927 lbs	20.0%	7	09-06-06
Total Load Deflection	2xL/286 (0.482")	n\a	83.9%	58	16-06-00
Live Load Deflection	2xL/619 (0.223")	n\a	58.2%	82	16-06-00
Total Neg. Defl.	L/999 (-0.099")	n\a	n\a	58	06-06-00
Max Defl.	-0.099"	n\a	n\a	58	06-06-00
Cant. Max Defl.	0.482"	n\a	48.2%	58	16-06-00
Span / Depth	10.6				



Bearin	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	3-1/2" x 7"	2935 lbs	19.5%	9.8%	Spruce-Pine-Fir
B0	Uplift		1180 l bs			
B1	Wall/Plate	5 - 1/2" x 7"	12407 I bs	52.4%	26.4%	Spruce-Pine-Fir

Cautions

Uplift of 1180 lbs found at bearing B0.

Long Cantilever: Sheathing required on bottom flange and adjacent back span or bracing designed by the design professional of record. Design professional of record must address uplift at supports.

NAIL ONE PLY TO ANOTHER WITH 3-1/2" SPIRAL NAILS @ STAGGERED IN 2 ROWS

PLUS SDW22634 SIMPSON WOOD SCREW@ 12" O/C, STAGGERED IN 2 ROWS.



BC CALC® Member Report



Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

PASSED

B18 (Floor Beam)

Dry | 1 span | No cant.

File name:

Specifier:

Designer:

318278

NL

March 20, 2020 13:19:03

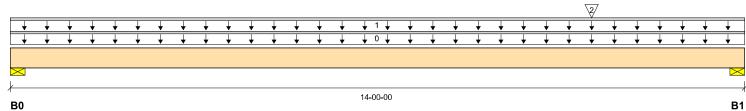
Build 7555

45147 (5004) Job name:

Pine Valley Address: Description: Second Floor Framing

City, Province, Postal Code: Vaughan, ON Builder: Gold Park

Code reports: CCMC 12472-R Company: Alpa Roof Trusses



Total Horizontal Product Length = 14-00-00

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B0, 3-1/2"	985 / 0	528 / 0	21 / 0	
B1. 3-1/2"	2828 / 0	1301 / 0	83 / 0	

Loa	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	14-00-00	Тор		12			00-00-00
1		Unf. Lin. (lb/ft)	L	00-00-00	14-00-00	Top	54	27			n∖a
2		Conc. Pt. (lbs)	L	11-01-00	11-01-00	Top	3057	1282	104		n∖a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	15452 ft-lbs	35392 ft-lbs	43.7%	1	11-01-00
End Shear	5785 lbs	14464 I bs	40.0%	1	12-08-10
Total Load Deflection	L/534 (0.304")	n\a	45.0%	11	07-08-09
Live Load Deflection	L/785 (0.207")	n\a	45.9%	15	07-08-09
Max Defl.	0.304"	n\a	30.4%	11	07-08-09
Span / Depth	13.7				

Bearing	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	3-1/2" x 3-1/2"	2158 lbs	28.6%	14.4%	Spruce-Pine-Fir
B1	Wall/Plate	3-1/2" x 3-1/2"	5951 lbs	79.0%	39.8%	Spruce-Pine-Fir



Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4

NAIL ONE PLY TO ANOTHER WITH 3-1/2" SPIRAL NAILS @ 12" O/C. STAGGERED IN 2 ROWS





B19 (Floor Beam)

File name:

Specifier:

318278

PASSED

BC CALC® Member Report

Dry | 1 span | No cant.

March 20, 2020 13:19:03

Build 7555

Job name: 45147 (5004)

Address: Pine Valley Description: First Floor Framing

City, Province, Postal Code: Vaughan, ON

Builder: Gold Park Designer: NL

Code reports: CCMC 12472-R Company: Alpa Roof Trusses

B0

B1

Total Horizontal Product Length = 08-03-00

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B0, 3-1/2"	687 / 0	530 / 0		
B1, 3-1/2"	688 / 0	530 / 0		

Loa	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	08-03-00	Тор		6			00-00-00
1		Unf. Area (lb/ft²)	L	00-00-00	08-03-00	Top	40	15			04-02-00
2		Unf. Lin. (lb/ft)	L	00-00-00	08-03-00	Top		60			n∖a

		Factored	Demand/		
Controls Summary	Factored Demand	Resistance	Resistance	Case	Location
Pos. Moment	3116 ft-lbs	17696 ft-lbs	17.6%	1	04-01-08
End Shear	1168 lbs	7232 I bs	16.1%	1	01-03-06
Total Load Deflection	L/999 (0.05")	n\a	n\a	4	04-01-08
Live Load Deflection	L/999 (0.028")	n\a	n\a	5	04-01-08
Max Defl.	0.05"	n\a	n\a	4	04-01-08
Span / Depth	7 9				

Bearing	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	3-1/2" x 1-3/4"	1694 I bs	45.0%	22.7%	Spruce-Pine-Fir
B1	Wall/Plate	3-1/2" x 1-3/4"	1694 I bs	45.0%	22.7%	Spruce-Pine-Fir

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4



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BC CALC®, BC FRAMER®, AJS™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,



B20 (Floor Beam)

Specifier:

NL



BC CALC® Member Report

Dry | 1 span | No cant.

March 20, 2020 13:19:03

Build 7555

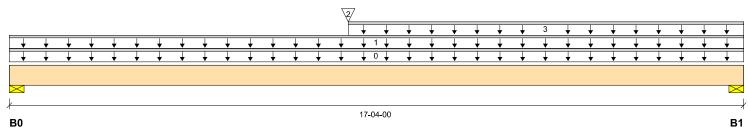
45147 (5004) Job name:

File name: 318278 Pine Valley First Floor Framing Address: Description:

City, Province, Postal Code: Vaughan, ON Builder:

Gold Park Designer:

Code reports: CCMC 12472-R Company: Alpa Roof Trusses



Total Horizontal Product Length = 17-04-00

Reaction Summary (Down / Uplift) (lbs)

Bearing Live Dead Snow Wind B0, 3-1/2" 839 / 0 772 / 0 B1, 3-1/2" 784 / 0 995 / 0

Lo	ad Summary						Live	Dead	Snow	Wind	Tributary
	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	17-04-00	Тор		12			00-00-00
1		Unf. Lin. (lb/ft)	L	00-00-00	17-04-00	Top	54	27			n\a
2		Conc. Pt. (Ibs)	L	08-00-00	08-00-00	Тор	687	530			n∖a
3		Unf. Lin. (lb/ft)	L	08-00-00	17-04-00	Тор		60			n∖a

		Factored	Demand/		
Controls Summary	Factored Demand	Resistance	Resistance	Case	Location
Pos. Moment	13121 ft-lbs	35392 ft-lbs	37.1%	1	08-00-00
End Shear	2158 lbs	14464 I bs	14.9%	1	16-00-10
Total Load Deflection	L/450 (0.45")	n\a	53.4%	4	08-07-09
Live Load Deflection	L/914 (0.221")	n\a	39.4%	5	08-06-01
Max Defl.	0.45"	n\a	45.0%	4	08-07-09
Span / Depth	17.1				

Beari	ng Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	3-1/2" x 3-1/2"	2222 lbs	29.5%	14.9%	Spruce-Pine-Fir
B1	Wall/Plate	3-1/2" x 3-1/2"	2420 lbs	32.1%	16.2%	Spruce-Pine-Fir



Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4

NAIL ONE PLY TO ANOTHER WITH 3-1/2" SPIRAL NAILS @ 12" O/C. STAGGERED IN 2 ROWS



B21 (Floor Beam)

File name:

Specifier:

Designer:

Company:

318278

Alpa Roof Trusses

NL

PASSED

March 20, 2020 13:19:03

BC CALC® Member Report

Dry | 1 span | No cant.

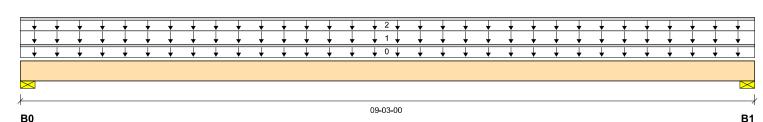
Build 7555

45147 (5004) Job name:

Pine Valley Address: Description: First Floor Framing

City, Province, Postal Code: Vaughan, ON Builder: Gold Park

CCMC 12472-R Code reports:



Total Horizontal Product Length = 09-03-00

Reaction Summary (Down / Uplift) (Ibs)

Bearing	Live	Dead	Snow	Wi
B0, 3-1/2"	740 / 0	675 / 0		
B1, 3-1/2"	740 / 0	675 / 0		

Loa	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	09-03-00	Тор		6			00-00-00
1		Unf. Area (Ib/ft²)	L	00-00-00	09-03-00	Top	40	20			04-00-00
2		Unf. Lin. (lb/ft)	L	00-00-00	09-03-00	Top		60			n∖a

		Factored	Demand/		
Controls Summary	Factored Demand	Resistance	Resistance	Case	Location
Pos. Moment	4082 ft-lbs	17696 ft-lbs	23.1%	1	04-07-08
End Shear	1413 lbs	7232 I bs	19.5%	1	01-03-06
Total Load Deflection	L/999 (0.084")	n\a	n\a	4	04-07-08
Live Load Deflection	L/999 (0.044")	n\a	n\a	5	04-07-08
Max Defl.	0.084"	n\a	n\a	4	04-07-08
Span / Depth	8.9				

Bearing	յ Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	3-1/2" x 1-3/4"	1954 I bs	51.9%	26.2%	Spruce-Pine-Fir
B1	Wall/Plate	3-1/2" x 1-3/4"	1954 I bs	51.9%	26.2%	Spruce-Pine-Fir

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4



Disclosure

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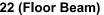
BC CALC®, BC FRAMER®, AJS™ ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,



BC CALC® Member Report

Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

B22 (Floor Beam) Dry | 1 span | No cant.



Specifier:

NL

March 20, 2020 13:19:03

PASSED

Build 7555

B1, 3-1/2"

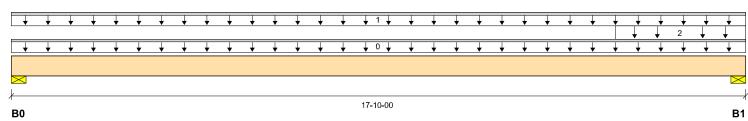
45147 (5004) Job name:

File name: 318278 Pine Valley Description: Second Floor Framing Address:

City, Province, Postal Code: Vaughan, ON Builder:

Gold Park Designer:

Code reports: CCMC 12472-R Company: Alpa Roof Trusses



Total Horizontal Product Length = 17-10-00

Reaction Summary (Down / Uplift) (Ibs)

591/0

Bearing	Live	Dead	Snow	Win
B0, 3-1/2"	271 / 0	778 / 0		

898 / 0

Loa	ad Summary						Live	Dead	Snow	Wind	Tributary
	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	17-10-00	Тор		12			00-00-00
1		Unf. Lin. (lb/ft)	L	00-00-00	17-10-00	Top	27	74			n∖a
2		Unf. Area (Ib/ft²)	L	14-08-00	17-10-00	Top	40	15			03-00-00

		Factored	Demand/		
Controls Summary	Factored Demand	Resistance	Resistance	Case	Location
Pos. Moment	4682 ft-lbs	23005 ft-lbs	20.4%	0	09-00-04
End Shear	1023 lbs	9401 l bs	10.9%	0	16-06-10
Total Load Deflection	L/800 (0.261")	n\a	30.0%	4	09-00-04
Live Load Deflection	L/999 (0.074")	n\a	n\a	5	09-02-12
Max Defl.	0.261"	n\a	26.1%	4	09-00-04
Span / Depth	17 6				

Beari	ing Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	3-1/2" x 3-1/2"	1090 lbs	22.2%	11.2%	Spruce-Pine-Fir
B1	Wall/Plate	3-1/2" x 3-1/2"	2009 lbs	26.7%	13.4%	Spruce-Pine-Fir



Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4

NAIL ONE PLY TO ANOTHER WITH 3-1/2" SPIRAL NAILS @ 12" O/C, STAGGERED IN 2 ROWS



B23 (Floor Beam)

Specifier:

Designer:

NL

PASSED

March 20, 2020 13:19:03

BC CALC® Member Report

Dry | 1 span | No cant.

Build 7555

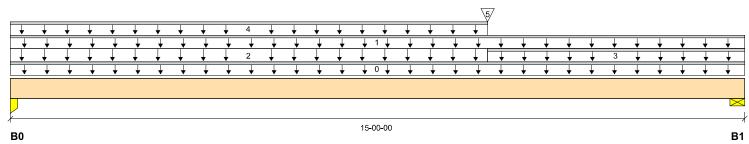
45147 (5004) Job name:

File name: 318278 Pine Valley Address: Description: Second Floor Framing

City, Province, Postal Code: Vaughan, ON Builder:

Gold Park

Code reports: CCMC 12472-R Company: Alpa Roof Trusses



Total Horizontal Product Length = 15-00-00

Reaction Summary (Down / Uplift) (Ibs)

Bearing	Live	Dead	Snow	Wind	
B0, 3"	2810 / 0	1806 / 0			
B1, 3-1/2"	1838 / 0	1458 / 0			

Load Summary						Live	Dead	Snow	Wind	Tributary
Tag Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0 Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	15-00-00	Тор		12			00-00-00
1	Unf. Lin. (lb/ft)	L	00-00-00	15-00-00	Тор	27	14			n∖a
2	Unf. Area (Ib/ft²)	L	00-00-00	09-09-00	Тор	40	15			09-00-00
3	Unf. Lin. (lb/ft)	L	09-09-00	15-00-00	Тор	27	14			n∖a
4	Unf. Lin. (lb/ft)	L	00-00-00	09-09-00	Тор		60			n∖a
5	Conc. Pt. (lbs)	L	09-09-00	09-09-00	Тор	591	898			n∖a

		Factored	Demand/		
Controls Summary	Factored Demand	Resistance	Resistance	Case	Location
Pos. Moment	23248 ft-lbs	35392 ft-lbs	65.7%	1	07-05-14
End Shear	5411 l bs	14464 I bs	37.4%	1	01-02-14
Total Load Deflection	L/273 (0.641")	n\a	87.9%	4	07-05-14
Live Load Deflection	L/472 (0.371")	n\a	76.4%	5	07-04-04
Max Defl.	0.641"	n\a	64.1%	4	07-05-14
Span / Depth	14.7				

Bearing	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Column	3" x 3-1/2"	6473 lbs	35.5%	50.5%	Spruce-Pine-Fir
B1	Wall/Plate	3-1/2" x 3-1/2"	4579 lbs	60.8%	30.6%	Spruce-Pine-Fir



Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4

NAIL ONE PLY TO ANOTHER WITH 3-1/2" SPIRAL NAILS @ 9" O/C, STAGGERED IN 2 ROWS



B24 (Floor Beam)

File name:

Specifier:

318278

PASSED

BC CALC® Member Report

Dry | 1 span | No cant.

March 20, 2020 13:19:03

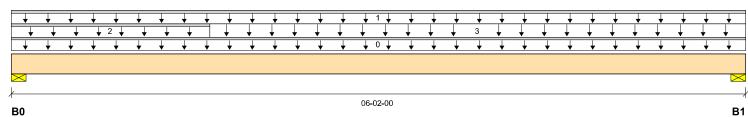
Build 7555

45147 (5004) Job name:

Pine Valley Address: Description: Second Floor Framing

City, Province, Postal Code: Vaughan, ON Builder: Gold Park

Designer: NL Alpa Roof Trusses CCMC 12472-R Company: Code reports:



Total Horizontal Product Length = 06-02-00

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B0, 3-1/2"	603 / 0	262 / 0		
B1, 3-1/2"	959 / 0	391 / 0		

Lo	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag	_	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	06-02-00	Тор		6			00-00-00
1		Unf. Lin. (lb/ft)	L	00-00-00	06-02-00	Top	27	14			n∖a
2		Unf. Lin. (lb/ft)	L	00-00-00	01-08-00	Top	27	14			n∖a
3		Unf. Area (lb/ft²)	L	01-08-00	06-02-00	Top	40	15			07-06-00

		Factored	Demand/		
Controls Summary	Factored Demand	Resistance	Resistance	Case	Location
Pos. Moment	2404 ft-lbs	17696 ft-lbs	13.6%	1	03-03-05
End Shear	1086 lbs	7232 lbs	15.0%	1	04-10-10
Total Load Deflection	L/999 (0.02")	n\a	n\a	4	03-01-08
Live Load Deflection	L/999 (0.014")	n\a	n\a	5	03-01-08
Max Defl.	0.02"	n\a	n\a	4	03-01-08
Span / Depth	5.8				

Ве	aring Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	3-1/2" x 1-3/4"	1232 lbs	32.7%	16.5%	Spruce-Pine-Fir
B1	Wall/Plate	3-1/2" x 1-3/4"	1927 lbs	51.1%	25.8%	Spruce-Pine-Fir

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4



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BC CALC®, BC FRAMER®, AJS™. ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,



BC CALC® Member Report

Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

R25 (Floor Roam)

File name:

Specifier:

318278

NL

B25 (Floor Beam)

Dry | 1 span | No cant.

March 20, 2020 13:19:03

PASSED

Build 7555

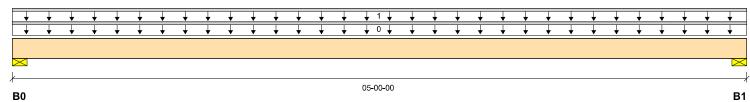
Job name: 45147 (5004)

Address: Pine Valley Description: Second Floor Framing

City, Province, Postal Code: Vaughan, ON

Builder: Gold Park Designer:

Code reports: CCMC 12472-R Company: Alpa Roof Trusses



Total Horizontal Product Length = 05-00-00

Reaction Summary (Down / Uplift) (lbs)

Reaction Summary (Down / Opint) (IDS)								
Bearing	Live	Dead	Snow	Wind				
B0, 3-1/2"	67 / 0	215 / 0						
B1, 3-1/2"	67 / 0	215 / 0						

Loa	Load Summary					Live	Dead	Snow	Wind	Tributary	
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	05-00-00	Тор		12			00-00-00
1		Unf. Lin. (lb/ft)	L	00-00-00	05-00-00	Top	27	74			n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	311 ft-lbs	23005 ft-lbs	1.3%	0	02-06-00
End Shear	147 l bs	9401 l bs	1.6%	0	01-03-06
Total Load Deflection	L/999 (0.001")	n\a	n\a	4	02-06-00
Live Load Deflection	L/999 (0")	n\a	n\a	5	02-06-00
Max Defl.	0.001"	n\a	n\a	4	02-06-00
Span / Depth	4.6				

Bearing	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	3-1/2" x 3-1/2"	301 lbs	6.1%	3.1%	Spruce-Pine-Fir
B1	Wall/Plate	3-1/2" x 3-1/2"	301 lbs	6.1%	3.1%	Spruce-Pine-Fir



Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4

NAIL ONE PLY TO ANOTHER WITH 3-1/2" SPIRAL NAILS @ 6" O/C, STAGGERED IN 2 ROWS





B26 (Floor Beam)

File name:

Specifier:

318278

NL

PASSED

March 20, 2020 13:19:03

BC CALC® Member Report

Dry | 1 span | No cant.

Build 7555

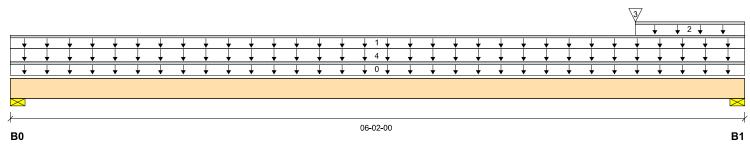
45147 (5004) Job name:

Pine Valley Address: Description: Second Floor Framing

City, Province, Postal Code: Vaughan, ON

Builder: Gold Park Designer:

Code reports: CCMC 12472-R Company: Alpa Roof Trusses



Total Horizontal Product Length = 06-02-00

Reaction Summary (Down / Uplift) (Ibs)

Bearing	Live	Dead	Snow	Wind	
B0, 3-1/2"	503 / 0	434 / 0			
B1. 3-1/2"	577 / 0	608 / 0			

Load Summary						Live	Dead	Snow	Wind	Tributary
Tag Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0 Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	06-02-00	Тор		12			00-00-00
1	Unf. Lin. (lb/ft)	L	00-00-00	06-02-00	Top		60			n∖a
2	Unf. Lin. (Ib/ft)	L	05-03-00	06-02-00	Top	27	14			n∖a
3	Conc. Pt. (lbs)	L	05-03-00	05-03-00	Тор	68	215			n∖a
4	Unf. Area (lb/ft²)	L	00-00-00	06-02-00	Top	40	15			04-00-00

		Factored	Demand/		
Controls Summary	Factored Demand	Resistance	Resistance	Case	Location
Pos. Moment	1787 ft-lbs	35392 ft-lbs	5.0%	1	03-02-06
End Shear	953 lbs	14464 I bs	6.6%	1	04-10-10
Total Load Deflection	L/999 (0.008")	n\a	n\a	4	03-01-11
Live Load Deflection	L/999 (0.004")	n\a	n\a	5	03-01-00
Max Defl.	0.008"	n\a	n\a	4	03-01-11
Span / Depth	5.8				

Bea	ring Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	3-1/2" x 3-1/2"	1296 lbs	17.2%	8.7%	Spruce-Pine-Fir
B1	Wall/Plate	3-1/2" x 3-1/2"	1626 lbs	21.6%	10.9%	Spruce-Pine-Fir



Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4

NAIL ONE PLY TO ANOTHER WITH 3-1/2" SPIRAL NAILS @ 6" O/C, STAGGERED IN 2 ROWS



PASSED

В1

March 20, 2020 13:19:03

B27 (Floor Beam)

File name:

Specifier:

318278

Wind

BC CALC® Member Report

Dry | 1 span | No cant.

Build 7555

B0

Job name: 45147 (5004)

Address: Pine Valley Description: Second Floor Framing

City, Province, Postal Code: Vaughan, ON Builder: Gold Park

Builder: Gold Park Designer: NL
Code reports: CCMC 12472-R Company: Alpa Roof Trusses

Total Horizontal Product Length = 13-04-00

Reaction Summary (Down / Uplift) (lbs)

 Bearing
 Live
 Dead
 Snow

 B0, 3-1/2"
 4726 / 0
 2890 / 0

 B1, 3-1/2"
 3739 / 0
 2434 / 0

Load Summary						Live	Dead	Snow	Wind	Tributary
Tag Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0 Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	13-04-00	Тор		18			00-00-00
1	Unf. Lin. (lb/ft)	L	00-00-00	01-04-00	Top	27	14			n\a
2	Unf. Area (lb/ft²)	L	01-04-00	08-00-00	Top	40	15			03-02-00
3	Unf. Lin. (lb/ft)	L	08-00-00	13-04-00	Top		60			n∖a
4	Unf. Area (Ib/ft²)	L	00-00-00	03-01-00	Тор	40	15			03-00-00
5	Unf. Area (lb/ft²)	L	03-01-00	13-04-00	Top	40	20			10-05-00
6	Conc. Pt. (lbs)	L	01-04-00	01-04-00	Top	603	262			n∖a
7	Conc. Pt. (lbs)	L	03-01-00	03-01-00	Top	1838	1458			n∖a
8	Conc. Pt. (lbs)	L	08-00-00	08-00-00	Тор	503	434			n∖a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	32131 ft-lbs	55212 ft-lbs	58.2%	1	06-01-14
End Shear	10295 lbs	21696 I bs	47.5%	1	01-03-06
Total Load Deflection	L/328 (0.471")	n\a	73.1%	4	06-05-09
Live Load Deflection	L/534 (0.289")	n\a	67.4%	5	06-05-09
Max Defl.	0.471"	n\a	47.1%	4	06-05-09
Span / Depth	13.0				

Bear	ing Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	3-1/2" x 5-1/4"	10701 lbs	94.7%	47.7%	Spruce-Pine-Fir
R1	Wall/Plate	3-1/2" x 5-1/4"	8652 lbs	76.5%	38.6%	Spruce-Pine-Fir



Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4

NAIL ONE PLY TO ANOTHER WITH 3-1/2" SPIRAL NAILS @ 5" O/C, STAGGERED IN 2 ROWS





B28 (Floor Beam)

File name:

Specifier:

Designer:

318278

NL

PASSED

BC CALC® Member Report

Dry | 1 span | No cant.

March 20, 2020 13:19:03

Build 7555

B1, 3-1/2"

Job name: 45147 (5004)

Address: Pine Valley Description: Second Floor Framing

City, Province, Postal Code: Vaughan, ON Builder: Gold Park

Code reports: CCMC 12472-R Company: Alpa Roof Trusses

B0

B1

Total Horizontal Product Length = 08-00-00

Reaction Summary (Down / Uplift) (Ibs)

3000 / 0

 Bearing
 Live
 Dead
 Snow
 Wind

 B0, 3-1/2"
 3000 / 0
 1788 / 0

1788 / 0

Lo	Load Summary							Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	08-00-00	Тор		12			00-00-00
1		Unf. Area (lb/ft²)	L	00-00-00	08-00-00	Top	40	20			18-09-00
2		Unf. Lin. (lb/ft)	L	00-00-00	08-00-00	Тор		60			n∖a

		Factored	Demand/		
Controls Summary	Factored Demand	Resistance	Resistance	Case	Location
Pos. Moment	11971 ft-lbs	35392 ft-lbs	33.8%	1	04-00-00
End Shear	4578 lbs	14464 I bs	31.7%	1	01-03-06
Total Load Deflection	L/999 (0.089")	n\a	n\a	4	04-00-00
Live Load Deflection	L/999 (0.056")	n\a	n\a	5	04-00-00
Max Defl.	0.089"	n\a	n\a	4	04-00-00
Span / Depth	7.6				

Bearing	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	3-1/2" x 3-1/2"	6735 lbs	89.4%	45.1%	Spruce-Pine-Fir
B1	Wall/Plate	3-1/2" x 3-1/2"	6735 lbs	89.4%	45.1%	Spruce-Pine-Fir



Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4

NAIL ONE PLY TO ANOTHER WITH 3-1/2" SPIRAL NAILS @ $\,6$ " O/C, STAGGERED IN 2 ROWS



B29 (Floor Beam)

File name:

Specifier:

Designer:

318278

NL



BC CALC® Member Report

Dry | 1 span | No cant.

March 20, 2020 13:19:03

Build 7555

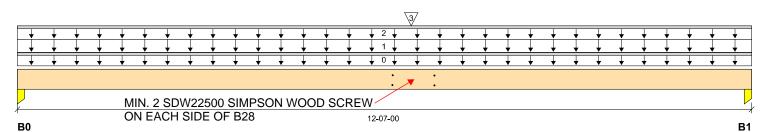
45147 (5004) Job name:

Pine Valley Address: Description: Second Floor Framing

City, Province, Postal Code: Vaughan, ON Builder:

Gold Park

Code reports: CCMC 12472-R Company: Alpa Roof Trusses



Total Horizontal Product Length = 12-07-00

Reaction Summary (Down / Unlift) (lbs)

Reaction Summary (Down / Opinit) (105)										
Bearing	Live	Dead	Snow	Wind						
B0, 3-1/2"	3791 / 0	2522 / 0								
B1, 3"	3991 / 0	2639 / 0								

Lo	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag		Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	12-07-00	Тор		18			00-00-00
1		Unf. Area (lb/ft²)	L	00-00-00	12-07-00	Тор	40	20			09-06-00
2		Unf. Lin. (lb/ft)	L	00-00-00	12-07-00	Тор		60			n\a
3		Conc. Pt. (lbs)	L	06-09-00	06-09-00	Тор	3000	1788			n∖a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	37038 ft-lbs	55212 ft-lbs	67.1%	1	06-09-00
End Shear	8164 l bs	21696 lbs	37.6%	1	11-04-02
Total Load Deflection	L/341 (0.428")	n\a	70.4%	4	06-04-14
Live Load Deflection	L/562 (0.26")	n\a	64.0%	5	06-04-14
Max Defl.	0.428"	n\a	42.8%	4	06-04-14
Span / Depth	12.3				

Bea	ring Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Column	3-1/2" x 5-1/4"	8838 lbs	27.7%	39.4%	Spruce-Pine-Fir
B1	Column	3" x 5-1/4"	9286 lbs	34.0%	48.3%	Spruce-Pine-Fir



Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4

NAIL ONE PLY TO ANOTHER WITH 3-1/2" SPIRAL NAILS @ 6" O/C, STAGGERED IN 2 ROWS





B30 (Floor Beam)

Specifier:

318278

NL

PASSED

March 20, 2020 13:19:03

BC CALC® Member Report

Dry | 1 span | No cant.

Build 7555

B1, 3-1/2"

45147 (5004) Job name:

File name: Address: Pine Valley Description: First Floor Framing

City, Province, Postal Code: Vaughan, ON Builder:

Gold Park Designer:

CCMC 12472-R Company: Code reports: Alpa Roof Trusses

12-07-00 B₀ **B1**

Total Horizontal Product Length = 12-07-00

Reaction Summary (Down / Uplift) (lbs)

1258 / 0

Bearing	Live	Dead	Snow	Wind
B0, 3-1/2"	1258 / 0	1045 / 0		

1045 / 0

Lo	Load Summary						Live	Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	12-07-00	Тор		6			00-00-00
1		Unf. Area (Ib/ft²)	L	00-00-00	12-07-00	Top	40	20			05-00-00
2		Unf. Lin. (lb/ft)	L	00-00-00	12-07-00	Тор		60			n∖a

		Factored	Demand/		
Controls Summary	Factored Demand	Resistance	Resistance	Case	Location
Pos. Moment	9327 ft-lbs	17696 ft-lbs	52.7%	1	06-03-08
End Shear	2543 lbs	7232 l bs	35.2%	1	01-03-06
Total Load Deflection	L/399 (0.364")	n\a	60.1%	4	06-03-08
Live Load Deflection	L/731 (0.199")	n\a	49.3%	5	06-03-08
Max Defl.	0.364"	n\a	36.4%	4	06-03-08
Span / Depth	12 3				

Bear	ing Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	3-1/2" x 1-3/4"	3193 lbs	84.7%	42.7%	Spruce-Pine-Fir
R1	Wall/Plate	3-1/2" v 1-3/4"	3103 lhe	84 7%	42 7%	Spruce-Pine-Fir

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4



Disclosure

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

BC CALC®, BC FRAMER®, AJS™ ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,



B31 (Floor Beam)

File name:

Specifier:

Designer:

318278

NL

PASSED

BC CALC® Member Report

Dry | 1 span | No cant.

March 20, 2020 13:19:03

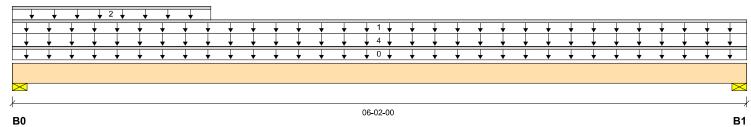
Build 7555

Job name: 45147 (5004)

Address: Pine Valley Description: First Floor Framing

City, Province, Postal Code: Vaughan, ON Builder: Gold Park

Code reports: CCMC 12472-R Company: Alpa Roof Trusses



Total Horizontal Product Length = 06-02-00

Reaction Summary (Down / Uplift) (lbs)

i toaotioii oai	a.				
Bearing	Live	Dead	Snow	Wind	
B0, 3-1/2"	533 / 0	517 / 0			
B1, 3-1/2"	498 / 0	420 / 0			

Lo	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	06-02-00	Тор		12			00-00-00
1		Unf. Lin. (lb/ft)	L	00-00-00	06-02-00	Top		60			n\a
2		Unf. Lin. (lb/ft)	L	00-00-00	01-08-00	Тор	27	74			n\a
4		Unf. Area (lb/ft²)	L	00-00-00	06-02-00	Top	40	15			04-00-00

		Factored	Demand/		
Controls Summary	Factored Demand	Resistance	Resistance	Case	Location
Pos. Moment	1719 ft-lbs	35392 ft-lbs	4.9%	1	02-11-11
End Shear	757 l bs	14464 I bs	5.2%	1	01-03-06
Total Load Deflection	L/999 (0.008")	n\a	n\a	4	03-00-14
Live Load Deflection	L/999 (0.004")	n\a	n\a	5	03-00-14
Max Defl.	0.008"	n\a	n\a	4	03-00-14
Span / Depth	5.8				

Bearing	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	3-1/2" x 3-1/2"	1446 I bs	19.2%	9.7%	Spruce-Pine-Fir
B1	Wall/Plate	3-1/2" x 3-1/2"	1273 lbs	16.9%	8.5%	Spruce-Pine-Fir



Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4

NAIL ONE PLY TO ANOTHER WITH 3-1/2" SPIRAL NAILS @ 12" O/C, STAGGERED IN 2 ROWS



BC CALC® Member Report

Triple 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

PASSED

B32 (Floor Beam)

File name:

Specifier:

Designer:

Company:

318278

Alpa Roof Trusses

Wind

NL

Dry | 1 span | No cant.

March 20, 2020 13:19:03

Build 7555

Job name: 45147 (5004)

Address: Pine Valley Description: First Floor Framing

City, Province, Postal Code: Vaughan, ON Builder: Gold Park

Code reports: CCMC 12472-R

Total Horizontal Product Length = 17-11-00

Reaction Summary (Down / Uplift) (Ibs)

 Bearing
 Live
 Dead
 Snow

 B0, 3-1/2"
 1744 / 0
 1317 / 0

 B1, 3-1/2"
 5029 / 0
 3312 / 0

Load Summary						Live	Dead	Snow	Wind	Tributary
Tag Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0 Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	17-11-00	Тор		18			00-00-00
1	Unf. Lin. (lb/ft)	L	00-00-00	17-11-00	Top	27	14			n∖a
2	Unf. Lin. (lb/ft)	L	04-04-00	10-10-00	Тор	27	14			n∖a
3	Unf. Area (Ib/ft²)	L	10-10-00	14-10-00	Тор	40	15			01-00-00
4	Conc. Pt. (lbs)	L	04-04-00	04-04-00	Тор	140	59			n∖a
5	Conc. Pt. (lbs)	L	10-10-00	10-10-00	Тор	140	59			n∖a
6	Unf. Lin. (lb/ft)	L	14-10-00	17-11-00	Top	27	14			n∖a
7	Conc. Pt. (lbs)	L	14-10-00	14-10-00	Тор	533	517			n∖a
8	Conc. Pt. (lbs)	L	14-10-00	14-10-00	Top	4726	2890			n∖a
9	Conc. Pt. (lbs)	L	00-05-00	00-05-00	Top	331	335			n∖a

		Factored	Demand/		
Controls Summary	Factored Demand	Resistance	Resistance	Case	Location
Pos. Moment	32701 ft-lbs	55212 ft-lbs	59.2%	1	14-10-00
End Shear	11505 lbs	21696 lbs	53.0%	1	16-07-10
Total Load Deflection	L/287 (0.73")	n\a	83.6%	4	09-09-13
Live Load Deflection	L/478 (0.438")	n\a	75.3%	5	09-09-13
Max Defl.	0.73"	n\a	73.0%	4	09-09-13
Span / Depth	17.6				

Bearii	ng Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	3-1/2" x 5-1/4"	4262 lbs	37.7%	19.0%	Spruce-Pine-Fir
R1	Ream	3-1/2" x 5-1/4"	11683 lbs	1.6%	52 1%	Steel



Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4





B33 (Floor Beam)

PASSED

March 20, 2020 13:19:03

BC CALC® Member Report

Dry | 1 span | No cant.

Build 7555

45147 (5004) Job name:

Pine Valley Address: Description: First Floor Framing

City, Province, Postal Code: Vaughan, ON Builder: Gold Park

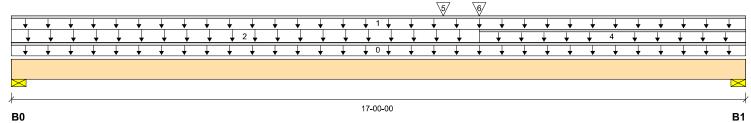
Code reports: CCMC 12472-R

318278 File name:

Specifier:

Designer:

NL Company: Alpa Roof Trusses



Total Horizontal Product Length = 17-00-00

Reaction Summary (Down / Uplift) (lbs)

Bearing Live Dead Snow Wind B0, 3-1/2" 3800 / 0 1909 / 0 B1, 3-1/2" 3029 / 0 1812 / 0

Load Summary						Live	Dead	Snow	Wind	Tributary
Tag Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0 Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	17-00-00	Тор		24			00-00-00
1	Unf. Lin. (lb/ft)	L	00-00-00	17-00-00	Top	27	14			n∖a
2	Unf. Area (lb/ft²)	L	00-00-00	10-10-00	Top	40	15			09-00-00
4	Unf. Lin. (lb/ft)	L	10-10-00	17-00-00	Тор	20	8			n∖a
5	Conc. Pt. (lbs)	L	10-00-00	10-00-00	Тор	603	245			n∖a
6	Conc. Pt. (lbs)	L	10-10-00	10-10-00	Top	1744	1317			n∖a

		Factored	Demand/		
Controls Summary	Factored Demand	Resistance	Resistance	Case	Location
Pos. Moment	39186 ft-lbs	73615 ft-lbs	53.2%	1	10-00-00
End Shear	7065 lbs	28927 lbs	24.4%	1	01-03-06
Total Load Deflection	L/299 (0.664")	n\a	80.3%	4	08-07-10
Live Load Deflection	L/463 (0.429")	n\a	77.7%	5	08-07-10
Max Defl.	0.664"	n\a	66.4%	4	08-07-10
Span / Depth	16.7				

Bearing	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	3-1/2" x 7"	8086 lbs	53.6%	27.1%	Spruce-Pine-Fir
B1	Wall/Plate	3-1/2" x 7"	6809 lbs	45.2%	22.8%	Spruce-Pine-Fir



Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4

NAIL ONE PLY TO ANOTHER WITH 3-1/2" SPIRAL NAILS @ 12" O/C,

STAGGERED IN 2 ROWS

PLUS SDW22634 SIMPSON WOOD SCREW @ 12" O/C, STAGGERED IN 2 ROWS



B34 (Floor Beam)

File name:

Specifier:

Designer:

318278

NL

PASSED

BC CALC® Member Report

Dry | 1 span | No cant.

March 20, 2020 13:19:03

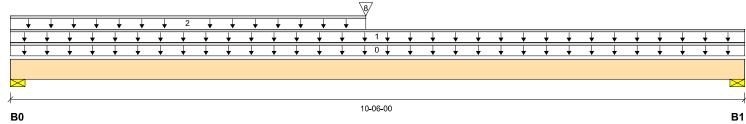
Build 7555

45147 (5004) Job name:

Pine Valley Description: First Floor Framing Address:

City, Province, Postal Code: Vaughan, ON Builder: Gold Park

Code reports: CCMC 12472-R Company: Alpa Roof Trusses



Total Horizontal Product Length = 10-06-00

Poaction Summary (Down / Unlift) (lbs)

Reaction Sui	ililialy (Dowli / O	piiit) (iba)			
Bearing	Live	Dead	Snow	Wind	
B0, 3-1/2"	507 / 0	731 / 0			
B1, 3-1/2"	417 / 0	678 / 0			

Lo	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	10-06-00	Top		12			00-00-00
1		Unf. Lin. (Ib/ft)	L	00-00-00	10-06-00	Top	27	74			n∖a
2		Unf. Lin. (lb/ft)	L	00-00-00	05-01-00	Тор	27	14			n∖a
8		Conc. Pt. (lbs)	L	05-01-00	05-01-00	Top	503	434			n∖a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	5469 ft-lbs	35392 ft-lbs	15.5%	1	05-01-00
End Shear	1410 l bs	14464 I bs	9.7%	1	01-03-06
Total Load Deflection	L/999 (0.066")	n\a	n∖a	4	05-02-09
Live Load Deflection	L/999 (0.028")	n\a	n∖a	5	05-02-09
Max Defl.	0.066"	n\a	n∖a	4	05-02-09
Span / Depth	10.1				

Bearing	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	3-1/2" x 3-1/2"	1674 lbs	22.2%	11.2%	Spruce-Pine-Fir
B1	Wall/Plate	3-1/2" x 3-1/2"	1472 lbs	19.5%	9.9%	Spruce-Pine-Fir



Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4

NAIL ONE PLY TO ANOTHER WITH 3-1/2" SPIRAL NAILS @ 12" O/C, STAGGERED IN 2 ROWS



B35 (Floor Beam)

PASSED

March 20, 2020 13:19:03

BC CALC® Member Report

Dry | 1 span | No cant.

Build 7555

45147 (5004) Job name:

Address: Pine Valley Description:

City, Province, Postal Code: Vaughan, ON Builder:

Gold Park

CCMC 12472-R Code reports:

318278

First Floor Framing

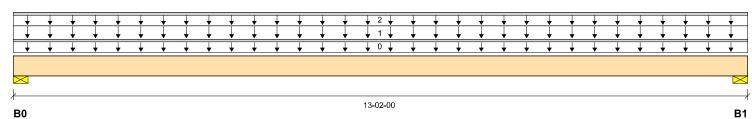
Specifier:

File name:

Designer: NL

Company: Alpa Roof Trusses

Wind



Total Horizontal Product Length = 13-02-00

Reaction Summary (Down / Uplift) (Ibs)

Bearing Live Dead Snow B0, 3-1/2" 658 / 0 681 / 0 B1, 3-1/2" 658 / 0 681 / 0

Loa	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	13-02-00	Тор		6			00-00-00
1		Unf. Area (lb/ft²)	L	00-00-00	13-02-00	Тор	40	15			02-06-00
2		Unf. Lin. (lb/ft)	L	00-00-00	13-02-00	Top		60			n∖a

		Factored	Demand/		
Controls Summary	Factored Demand	Resistance	Resistance	Case	Location
Pos. Moment	5640 ft-lbs	17696 ft-lbs	31.9%	1	06-07-00
End Shear	1481 l bs	7232 I bs	20.5%	1	01-03-06
Total Load Deflection	L/624 (0.245")	n\a	38.5%	4	06-07-00
Live Load Deflection	L/999 (0.12")	n\a	n\a	5	06-07-00
Max Defl.	0.245"	n\a	24.5%	4	06-07-00
Span / Depth	12.8				

Beari	ing Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	3-1/2" x 1-3/4"	1839 lbs	48.8%	24.6%	Spruce-Pine-Fir
R1	Wall/Plate	3-1/2" v 1-3/4"	1830 lhe	48.8%	24.6%	Spruce-Pine-Fir

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4



Disclosure

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

BC CALC®, BC FRAMER®, AJS™ ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,



BC CALC® Member Report



Single 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

B36 (Floor Beam)

Dry | 1 span | No cant.

Specifier:

Designer:

NL

Wind

March 20, 2020 13:19:03

PASSED

Build 7555

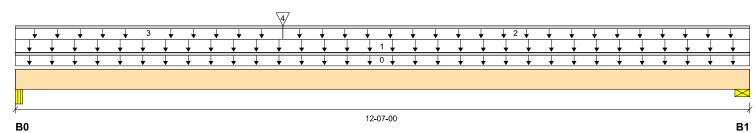
45147 (5004) Job name:

File name: 318278 Pine Valley Address: Description: First Floor Framing

City, Province, Postal Code: Vaughan, ON Builder: Gold Park

CCMC 12472-R

Company: Alpa Roof Trusses Code reports:



Total Horizontal Product Length = 12-07-00

Reaction Summary (Down / Uplift) (lbs)

Bearing Live Dead Snow B0, 3-1/2" 1552 / 0 1191 / 0 B1, 3-1/2" 1285 / 0 1138 / 0

Lo	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	12-07-00	Тор		6			00-00-00
1		Unf. Area (Ib/ft²)	L	00-00-00	12-07-00	Top	40	20			04-01-00
2		Unf. Lin. (lb/ft)	L	04-07-00	12-07-00	Тор		60			n∖a
3		Unf. Lin. (lb/ft)	L	00-00-00	04-07-00	Top	27	14			n∖a
4		Conc. Pt. (lbs)	L	04-07-00	04-07-00	Top	658	681			n∖a

		Factored	Demand/		
Controls Summary	Factored Demand	Resistance	Resistance	Case	Location
Pos. Moment	12303 ft-lbs	17696 ft-lbs	69.5%	1	04-09-09
End Shear	3288 lbs	7232 I bs	45.5%	1	01-03-06
Total Load Deflection	L/315 (0.462")	n\a	76.1%	4	06-01-14
Live Load Deflection	L/588 (0.248")	n\a	61.3%	5	06-01-14
Max Defl.	0.462"	n\a	46.2%	4	06-01-14
Span / Depth	12.3				

Bearing	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Beam	3-1/2" x 1-3/4"	3817 I bs	1.6%	51.1%	Steel
B1	Wall/Plate	3-1/2" x 1-3/4"	3350 lbs	88.9%	44.8%	Spruce-Pine-Fir

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4



Disclosure

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

BC CALC®, BC FRAMER®, AJS™ ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,

SE007857



BC CALC® Member Report



B37 (Floor Beam)

NL

Dry | 1 span | No cant.

PASSED

March 20, 2020 13:19:03

Build 7555

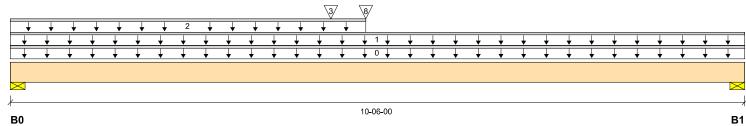
Job name: 45147 (5004)

File name: 318278 Pine Valley Description: First Floor Framing Address:

City, Province, Postal Code: Vaughan, ON

Specifier: Builder: Gold Park Designer:

Code reports: CCMC 12472-R Company: Alpa Roof Trusses



Total Horizontal Product Length = 10-06-00

Poaction Summary (Down / Unlift) (lbs)

ixeaction Sui	ililialy (Dowli / O	piiit) (iba)			
Bearing	Live	Dead	Snow	Wind	
B0, 3-1/2"	880 / 0	1116 / 0			
B1, 3-1/2"	702 / 0	973 / 0			

Load Summary						Live	Dead	Snow	Wind	Tributary
Tag Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0 Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	10-06-00	Тор		12			00-00-00
1	Unf. Lin. (Ib/ft)	L	00-00-00	10-06-00	Top	27	74			n\a
2	Unf. Lin. (lb/ft)	L	00-00-00	05-01-00	Top	27	14			n∖a
3	Conc. Pt. (lbs)	L	04-07-00	04-07-00	Тор	658	681			n∖a
8	Conc. Pt. (lbs)	L	05-01-00	05-01-00	Тор	503	434			n∖a

		Factored	Demand/		
Controls Summary	Factored Demand	Resistance	Resistance	Case	Location
Pos. Moment	9663 ft-lbs	35392 ft-lbs	27.3%	1	04-07-00
End Shear	2451 lbs	14464 I bs	16.9%	1	01-03-06
Total Load Deflection	L/999 (0.115")	n\a	n\a	4	05-01-00
Live Load Deflection	L/999 (0.052")	n\a	n\a	5	05-01-00
Max Defl.	0.115"	n\a	n\a	4	05-01-00
Span / Depth	10.1				

Bearing	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	3-1/2" x 3-1/2"	2715 lbs	36.0%	18.2%	Spruce-Pine-Fir
B1	Wall/P l ate	3-1/2" x 3-1/2"	2269 lbs	30.1%	15.2%	Spruce-Pine-Fir



Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4

NAIL ONE PLY TO ANOTHER WITH 3-1/2" SPIRAL NAILS @ 12" O/C, STAGGERED IN 2 ROWS



Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP



B38 (Floor Beam)

File name:

Specifier:

318278

NL

BC CALC® Member Report

Dry | 1 span | No cant.

March 20, 2020 13:19:03

Build 7555

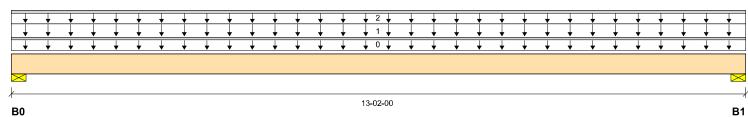
45147 (5004) Job name:

Pine Valley Description: First Floor Framing Address:

City, Province, Postal Code: Vaughan, ON Builder:

Gold Park Designer:

Code reports: CCMC 12472-R Company: Alpa Roof Trusses



Total Horizontal Product Length = 13-02-00

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind	
B0, 3-1/2"	1053 / 0	985 / 0			
B1, 3-1/2"	1053 / 0	985 / 0			

Loa	ad Summary						Live	Dead	Snow	Wind	Tributary
	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	13-02-00	Тор		10			00-00-00
1		Unf. Area (Ib/ft²)	L	00-00-00	13-02-00	Тор	40	20			04-00-00
2		Unf. Lin. (lb/ft)	L	00-00-00	13-02-00	Top		60			n∖a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	8621 ft-lbs	23220 ft-lbs	37.1%	1	06-07-00
End Shear	2349 lbs	11571 I bs	20.3%	1	01-01-00
Total Load Deflection	L/420 (0.363")	n\a	57.2%	4	06-07-00
Live Load Deflection	L/812 (0.188")	n\a	44.3%	5	06-07-00
Max Defl.	0.363"	n\a	36.3%	4	06-07-00
Span / Depth	16.1				

Bearing	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	3-1/2" x 3-1/2"	2811 lbs	37.3%	18.8%	Spruce-Pine-Fir
B1	Wall/Plate	3-1/2" x 3-1/2"	2811 lbs	37.3%	18.8%	Spruce-Pine-Fir



Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4

NAIL ONE PLY TO ANOTHER WITH 3-1/2" SPIRAL NAILS @ 12" O/C, STAGGERED IN 2 ROWS



Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

Designer:

B39 (Floor Beam) Dry | 1 span | No cant.

PASSED

March 20, 2020 13:19:03

BC CALC® Member Report Build 7555

45147 (5004) Job name:

318278 File name: Pine Valley Address: Description: Second Floor Framing

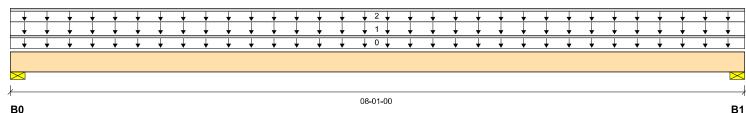
City, Province, Postal Code: Vaughan, ON Builder: Gold Park

Code reports: CCMC 12472-R

Specifier:

Company: Alpa Roof Trusses

NL



Total Horizontal Product Length = 08-01-00

Reaction Summary (Down / Uplift) (lbs)

Bearing Live Dead Snow Wind B0, 3-1/2" 1638 / 0 2694 / 0 B1, 3-1/2" 1638 / 0 2694 / 0

Loa	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	08-01-00	Top		12			00-00-00
1		Unf. Area (lb/ft²)	L	00-00-00	08-01-00	Top	40	20			16-08-00
2		Unf. Lin. (lb/ft)	L	00-00-00	08-01-00	Top		60			n∖a

		Factored	Demand/		
Controls Summary	Factored Demand	Resistance	Resistance	Case	Location
Pos. Moment	10950 ft-lbs	35392 ft-lbs	30.9%	1	04-00-08
End Shear	4159 l bs	14464 I bs	28.8%	1	01-03-06
Total Load Deflection	L/999 (0.083")	n\a	n\a	4	04-00-08
Live Load Deflection	L/999 (0.052")	n\a	n\a	5	04-00-08
Max Defl.	0.083"	n\a	n\a	4	04-00-08
Span / Depth	7 7				

Bearing	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	3-1/2" x 3-1/2"	6090 l bs	80.8%	40.7%	Spruce-Pine-Fir
B1	Wall/Plate	3-1/2" x 3-1/2"	6090 lbs	80.8%	40.7%	Spruce-Pine-Fir



Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

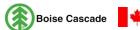
Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4

NAIL ONE PLY TO ANOTHER WITH 3-1/2" SPIRAL NAILS @ 9" O/C, STAGGERED IN 2 ROWS



Triple 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

B40 (Floor Beam)

File name:

Specifier:

Designer:

318278

NL



BC CALC® Member Report

Dry | 1 span | No cant.

March 20, 2020 13:19:03

Build 7555

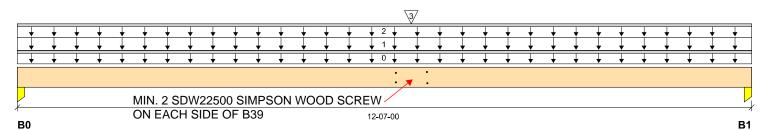
Job name: 45147 (5004)

Address: Pine Valley Description: Second Floor Framing

City, Province, Postal Code: Vaughan, ON

Builder: Gold Park

Code reports: CCMC 12472-R Company: Alpa Roof Trusses



Total Horizontal Product Length = 12-07-00

Reaction Summary (Down / Uplift) (lbs)

 Bearing
 Live
 Dead
 Snow
 Wind

 B0, 3-1/2"
 3649 / 0
 2452 / 0

 B1, 3"
 3827 / 0
 2559 / 0

L	oad Summary						Live	Dead	Snow	Wind	Tributary
_T:	ag Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	12-07-00	Тор		18			00-00-00
1		Unf. Area (Ib/ft²)	L	00-00-00	12-07-00	Top	40	20			09-06-00
2		Unf. Lin. (lb/ft)	L	00-00-00	12-07-00	Top		60			n∖a
3		Conc. Pt. (lbs)	L	06-09-00	06-09-00	Тор	2694	1638			n∖a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	35082 ft-lbs	55212 ft-lbs	63.5%	1	06-09-00
End Shear	7818 I bs	21696 lbs	36.0%	1	11-04-02
Total Load Deflection	L/358 (0.408")	n\a	67.1%	4	06-04-14
Live Load Deflection	L/593 (0.246")	n\a	60.7%	5	06-04-14
Max Defl.	0.408"	n\a	40.8%	4	06-04-14
Span / Depth	12.3				

Beari	ing Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Column	3-1/2" x 5-1/4"	8538 lbs	26.8%	38.1%	Spruce-Pine-Fir
B1	Column	3" x 5-1/4"	8939 lbs	32.7%	46.5%	Spruce-Pine-Fir



Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4

NAIL ONE PLY TO ANOTHER WITH 3-1/2" SPIRAL NAILS @ 6" O/C, STAGGERED IN 2 ROWS



Maximum Floor Spans - M2.1, L/360

Design Criteria

Spans: Simple span

Live load = 40 psf and dead load = 20 psf
Deflection limits: L/360 under live load and L/240 under total load

Sheathing: 5/8 in. nailed-glued oriented strand board (OSB) sheathing



Maximum Floor Spans

			В	are			1/2 in. gyr	osum ceiling	
Joist depth	Joist series		On cent	re spacing		On centre spacing			
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	15'-1"	14'-3"	13'-10"	-	15'-7"	14'-9"	14'-3"	_
0.4/0"	NI-40x	16'-2"	15'-3"	14'-8"	-	16'-7"	15'-8"	15'-1"	-
9-1/2"	NI-60	16'-4"	15'-4"	14'-10"	-	16'-9"	15'-9"	15'-3"	-
	NI-80	17'-3"	16'-3"	15'-8"	-	17'-8"	16'-7"	16'-0"	_
	NI-20	17'-0"	16'-0"	15'-6"	-	17'-6"	16'-7"	16'-0"	_
	NI-40x	18'-2"	17'-1"	16'-6"	-	18'-9"	17'-6"	16'-11"	_
11-7/8"	NI-60	18'-5"	17'-3"	16'-8"	-	19'-0"	17'-8"	17'-1"	_
	NI-80	19'-9"	18'-3"	17'-7"	-	20'-4"	18'-10"	18'-0"	-
	NI-90	20'-2"	18'-8"	17'-10"	-	20'-9"	19'-2"	18'-4"	_
	NI-40x	20'-1"	18'-8"	17'-10"	-	20'-10"	19'-4"	18'-6"	-
14"	NI-60	20'-6"	18'-11"	18'-2"	-	21'-2"	19'-8"	18'-9"	_
14	NI-80	21'-11"	20'-3"	19'-4"	-	22'-7"	20'-11"	20'-0"	_
	NI-90	22'-5"	20'-8"	19'-9"	-	23'-0"	21'-4"	20'-4"	_
	NI-60	22'-4"	20'-8"	19'-9"	-	23'-1"	21'-5"	20'-6"	_
16"	NI-80	23'-11"	22'-1"	21'-1"	-	24'-8"	22'-10"	21'-9"	-
	NI-90	24'-5"	22'-6"	21'-6"	_	25'-1"	23'-2"	22'-2"	_

		Mi	d-span blocking	g with 1x4 inch s	trap	Mid-sp	an blocking an	d 1/2 in. gypsum	ceiling
Joist depth	Joist series	On centre spacing				On centre spacing			
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	16'-11"	15'-5"	14'-6"	-	17'-1"	15'-5"	14'-6"	-
0.4/0"	NI-40x	17'-11"	17'-0"	16'-5"	-	18'-5"	17'-4"	16'-7"	-
9-1/2"	NI-60	18'-2"	17'-1"	16'-6"	-	18'-8"	17'-6"	16'-10"	-
	NI-80	19'-5"	18'-0"	17'-5"	-	19'-10"	18'-5"	17'-8"	-
	NI-20	19'-7"	18'-2"	17'-6"	-	20'-3"	18'-8"	17'-6"	-
	NI-40x	21'-1"	19'-7"	18'-8"	-	21'-8"	20'-2"	19'-0"	-
11-7/8"	NI-60	21'-4"	19'-9"	18'-11"	-	21'-11"	20'-5"	19'-6"	-
	NI-80	22'-9"	21'-1"	20'-2"	-	23'-3"	21'-8"	20'-8"	-
	NI-90	23'-3"	21'-6"	20'-6"	-	23'-9"	22'-0"	21'-0"	-
	NI-40x	23'-8"	21'-11"	20'-11"	-	24'-4"	22'-8"	20'-11"	-
14"	NI-60	24'-0"	22'-3"	21'-3"	-	24'-8"	22'-11"	21'-11"	-
14	NI-80	25'-7"	23'-9"	22'-7"	-	26'-2"	24'-4"	23'-3"	-
	NI-90	26'-1"	24'-2"	23'-0"	-	26'-8"	24'-9"	23'-7"	-
	NI-60	26'-5"	24'-6"	23'-5"	-	27'-2"	25'-3"	24'-2"	-
16"	NI-80	28'-2"	26'-1"	24'-10"	-	28'-10"	26'-9"	25'-6"	-
	NI-90	28'-8"	26'-6"	25'-3"	_	29'-3"	27'-2"	25'-11"	_

Notes

- 1. The tabulated clear spans are based on CSA O86-14 and NBC 2015, and are applicable to residential floor construction meeting the above design criteria.
- 2. For multiple-span applications, the end spans shall be 40% or more of the adjacent span.
- 3. Minimum bearing length shall be 1-3/4 inch for end bearings, and 3-1/2 inches for intermediate bearings.
- 4. Bearing stiffeners are not required when I-joists are used in accordance with this table, except as required for hangers.
- 5. Nordic I-joists are listed in CCMC Evaluation Report 13032-R and APA Product Report PR-L274C.



Maximum Floor Spans - M4.1, L/360

Design Criteria

Spans: Simple span

Loads: Live load = 40 psf and dead load = 20 psf
Deflection limits: L/360 under live load and L/240 under total load

Sheathing: 3/4 in. nailed-glued oriented strand board (OSB) sheathing



Maximum Floor Spans

			В	are			1/2 in. gyլ	osum ceiling	
Joist depth	Joist series	Joist series On centre spacing			On centre spacing				
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	15'-11"	15'-0"	14'-6"	13'-5"	16'-5"	15'-5"	14'-6"	13'-5"
0.4/0"	NI-40x	17'-0"	16'-0"	15'-5"	14'-10"	17'-5"	16'-5"	15'-10"	14'-11'
9-1/2"	NI-60	17'-2"	16'-2"	15'-7"	14'-11"	17'-7"	16'-7"	16'-0"	15'-4"
	NI-80	18'-3"	17'-1"	16'-5"	15'-9"	18'-8"	17'-5"	16'-9"	16'-1"
	NI-20	17'-11"	16'-11"	16'-3"	15'-8"	18'-7"	17'-5"	16'-10"	16'-1"
	NI-40x	19'-4"	17'-11"	17'-3"	16'-7"	19'-11"	18'-6"	17'-9"	17'-0"
11-7/8"	NI-60	19'-7"	18'-2"	17'-6"	16'-9"	20'-2"	18'-9"	17'-11"	17'-2"
	NI-80	21'-1"	19'-6"	18'-6"	17'-7"	21'-7"	20'-0"	19'-0"	18'-0"
	NI-90	21'-6"	19'-10"	18'-11"	17'-11"	22'-0"	20'-4"	19'-5"	18'-4"
	NI-40x	21'-5"	19'-11"	18'-11"	18'-0"	22'-1"	20'-7"	19'-7"	18'-7"
4.411	NI-60	21'-10"	20'-2"	19'-3"	18'-3"	22'-6"	20'-10"	19'-11"	18'-10
14"	NI-80	23'-5"	21'-7"	20'-7"	19'-5"	24'-0"	22'-3"	21'-2"	20'-0"
	NI-90	23'-10"	22'-1"	21'-0"	19'-10"	24'-5"	22'-7"	21'-6"	20'-4"
	NI-60	23'-9"	22'-0"	21'-0"	19'-10"	24'-6"	22'-9"	21'-8"	20'-7"
16"	NI-80	25'-6"	23'-7"	22'-5"	21'-2"	26'-2"	24'-3"	23'-1"	21'-10
	NI-90	26'-0"	24'-0"	22'-10"	21'-6"	26'-7"	24'-8"	23'-5"	22'-2"

		Mi	d-span blocking	g with 1x4 inch	strap	Mid-s	pan blocking an	d 1/2 in. gypsui	m ceiling
Joist depth	Joist series	On centre spacing				On centre spacing			
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	17'-1"	15'-5"	14'-6"	13'-5"	17'-1"	15'-5"	14'-6"	13'-5"
0.4/0"	NI-40x	18'-8"	17'-6"	16'-7"	14'-11"	19'-2"	17'-8"	16'-7"	14'-11"
9-1/2"	NI-60	18'-11"	17'-8"	16'-10"	15'-7"	19'-5"	18'-0"	16'-10"	15'-7"
	NI-80	20'-3"	18'-10"	17'-11"	17'-2"	20'-8"	19'-3"	18'-4"	17'-5"
	NI-20	20'-3"	18'-8"	17'-6"	16'-1"	20'-7"	18'-8"	17'-6"	16'-1"
	NI-40x	21'-10"	20'-4"	19'-0"	17'-0"	22'-5"	20'-10"	19'-0"	17'-0"
11-7/8"	NI-60	22'-1"	20'-7"	19'-8"	18'-7"	22'-8"	21'-2"	20'-3"	18'-8"
	NI-80	23'-8"	22'-0"	20'-11"	19'-10"	24'-1"	22'-6"	21'-6"	20'-4"
	NI-90	24'-1"	22'-5"	21'-4"	20'-2"	24'-7"	22'-11"	21'-10"	20'-8"
	NI-40x	24'-5"	22'-9"	20'-11"	18'-8"	25'-1"	22'-11"	20'-11"	18'-8"
4.411	NI-60	24'-10"	23'-2"	22'-1"	20'-10"	25'-6"	23'-10"	22'-9"	21'-4"
14"	NI-80	26'-6"	24'-8"	23'-6"	22'-2"	27'-1"	25'-3"	24'-1"	22'-9"
	NI-90	27'-0"	25'-1"	23'-11"	22'-7"	27'-6"	25'-8"	24'-6"	23'-2"
	NI-60	27'-3"	25'-5"	24'-3"	22'-11"	28'-0"	26'-2"	25'-0"	23'-1"
16"	NI-80	29'-1"	27'-1"	25'-9"	24'-4"	29'-8"	27'-9"	26'-5"	25'-0"
	NI-90	29'-7"	27'-6"	26'-2"	24'-9"	30'-2"	28'-2"	26'-10"	25'-5"

Notes

- 1. The tabulated clear spans are based on CSA O86-14 and NBC 2015, and are applicable to residential floor construction meeting the above design criteria.
- 2. For multiple-span applications, the end spans shall be 40% or more of the adjacent span.
- 3. Minimum bearing length shall be 1-3/4 inch for end bearings, and 3-1/2 inches for intermediate bearings.
- 4. Bearing stiffeners are not required when I-joists are used in accordance with this table, except as required for hangers.
- 5. Nordic I-joists are listed in CCMC Evaluation Report 13032-R and APA Product Report PR-L274C.

The construction details for residential designs are prone to changes.

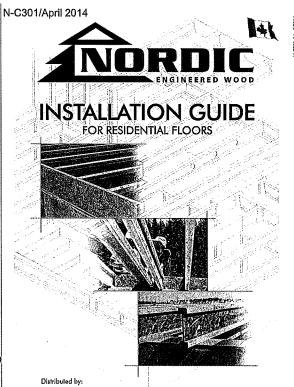
Details released after April 2014 supersedes N-C301

Installation must comply with latest documentation on I-Joist and other Nordic products from the http://nordic.ca/

This document does not constitute a record of the structural integrity of the building nor suitability of the design assumptions made. Nordic Structures is responsible only for the structural adequacy of its component based on the design criteria and loadings shown on the calculation sheets.

(Nordic Request 1810-095)





SAFETY AND CONSTRUCTION PRECAUTIONS





i-joists are not stable until completely installed, and will not carry any load until fully braced and sheafted.

Avoid Accidents by Following these Important Guidelines:

- Wolfd Actionins by rendering international moderation between the first plants and in the first plants and in the first plants and plants plants and so were limited in supports and a local-bearing well is planted at that location, blocking will be required at the interior support.
- Whan the building is complated, the floor steathing will provide lateral support for the top flanges of the I-lots. Until this sheathing is applied, temporary bracing, office called struts, or temporary sheathing must be applied to prevent I-joist rollover or budding.
 - Bempartay President of successing.

 Bempartay President of sets and the last Inch minimum, at Issus 8 feet long and spaced no more than 8 feet on centre, and must be socured with a minimum of two 2-1/27 mails featured to the top participa of each felsion. Half the brocking to a lasted restrict in the end of each bay, Lap ends of adjoining bracking over all feats two lights.
- Or, sheathing (temporary or permanent) can be nailed to the top flange of the first 4 feet of 1-joints at the end of the bay.
- 3. For cantilevered I-joists, brace top and bottom flanges, and brace ends with closure panels, rim board, or cross-bridging.
- 4. Install and fully nail permanent sheathing to each I-joist before placing loads on the floor system. Then, stack building materials over beams or walls only.

5. Never Install a damaged Lipist.

proper storage or installation, kalkure to follow applicable building codes, kalkure to follow span tatings for orde: I-joists, failure to follow allowable hole sizes and locations, or failure to are web stifteners when required in result is realow accidents. Follow interes installation, guidalines corellolly.



- Store, stock, and handle t-joists vertically and level only.
- Always stack and handle Hjoists in the upright position only.
- 4. Do not store I-joists in direct contact with the ground and/or flatwise. 5. Protect I-juists from weather, and use spacers to separate bundles.
- 6. Bundled units should be kept intact until time of installation.
- When handling I-joists with a crone on the job site, take a few simple precautions to prevent damage to the I-joists and injury to your work crew.
 - ■Pick I-joists in bundles as shipped by the supplier.
 - "Orient the bundles so that the webs of the 1-joists are vertical.
 - \bullet Pick the bundles at the 5% points, using a spreader bar if necessary.
- Do not handle l-joists in a horizontal orientation
- 9. NEVER USE OR TRY TO REPAIR A DAMAGED I-JOIST.



INSTALLING NORDIC I-JOISTS

- Before laying out floor system components, verify that I -joist flange widths treatch hunger widths. If not, contact your supplier.
- 2. Except for cutting to length, I-joist flanges should never be cut, drilled, or notched.
- 3. Install I-joists so that top and bottom flanges are within 1/2 inch of true vertical alignment
- 4. I-joints must be anchored securely to supports before floor shouthing is attached, and supports for multiple-spain joints must be level.
- 5. Minimum bearing lengths: 1-3/4 inches for end bearings and 3-1/2 inches for intermediate bearings.
- 6. When using hangers, seat I-joists firmly in hanger bottoms to minimize settlement.
- 7. Leave a 1/16-inch gap between the I-joist end and a header.
- 8. Concentrated loads greater than those first can normally be expected in residential construction should only be applied to the top surface of first loop fittings. Normal concentrated loads include track lighting fatures, auctio equipment and security conterars. Never supposed unaution of reacy loads from the 1-joil's cholonit fittings. Whenever possible, suspend off concentrated loads from the top of the 1-joils. Or, attach file load to blocking that has been securely fastened to the 1-joils walbs.
- Never install Lights where they will be permonerally exposed to weather, or where they will remain in direct contact with control or material.
- 10. Restrain ends of floor joists to prevent rollover. Use rim board, rim joists or I-joist blocking panels.
- 11. For I-joists installed over and beneath bearing walls, use full depth blocking panels, rim board, or squash blocks (cripple members) to transfer gravity loads through the floor system to the wall or foundation below.
- 12. Due to shrinkage, common framing lumber set on edge may nover be used as blocking or sim boards. I-joist blacking panels or other engineered wood products such as rim board must be cut to fit between the I-joists, and on I-joist-compatible depth relaceded.
- 13. Provide permonent lateral support of the bottom flange of all Lights at interior supports of multiple-span loists. Strailarly, support like bottom flange of all canflevered Lights of the end support need to the cantillover extension. In the completed structure, the gypsum wollboard calling provides this lateral support. Until the final finished ceiling is applied, temporary bracking or stroit must be used.
- 14. If square-edge ponels are used, edges must be supported between I-joists with 2x4 blocking. Glue ponels to blocking to minimize squeeks. Blocking is not required under structural flaits flooring, such as wood strip flooring, or if a separate underlayment layer is fustalled.
- 15. Nail spacing: Space nails installed to the flange's top face in accordance with the applicable building code requirem approved building plans.

TYPICAL NORDIC I-JOIST FLOOR FRAMING AND CONSTRUCTION DETAILS Figures 3, 4 or 5 Some framing requirements such as arection bracing and blocking panels have been omitted for clarity. tioles may be cut in web for plumbing, wiring and duct work. See Tables 1, 2 and Figure 7. Θ NOTE: Never cut or Nordic Lam or Structural (B)(B) ① 10 (1) 00000(1) (1)

All nails shown in the above datalls are assumed to be common wire nails unless otherwise noted. 3' (0.122' dis.) common spind rails may be substituted for 2-1/2' (0.126' dis.) common spind rails may be substituted for 2-1/2' (0.126' dis.) common spind enails. Framing humber assumed to be Spruce-Pino-Fir No. 2 or better, individual components not on thewn to scale for clarity.

(H)



Blocking Panel or Rim Joist	Maximum Factored Uniform Vertical Load* (plf)
NI Joists	3,300

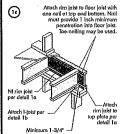
*The uniform vertical load is limited to a joist depth of 16 inches or less and is based on standard term load duration is shall not be used in the design of a bending member, such as joist, header, or rafter. For concentrated vertical load transfer, see detail 1d.



- Attach rim board to top plate using 2-1/2* wire or spiral toe-nails at 6" o.c To avoid splitting flange, start nails at least 1-1/2* from end of Ljoist. Nails avy be driven at an arryle to Minimum bearing length shall be 1-3/4" for the end bearings, and 3-1/2" for the intermediate bearings when applicable.

Maximum Factored Uniform Vortical Load* (plf)

"The uniform vertical load is limited to a rim board depth of 16 inche or loss and is based on standard torm load duration. If shall not bused in the design of a bending member, such as joist, header, or ratios. For concentrated vertical load transfer, see detail 1 d.







Maximum Factored Vertical per Pair of Saugsh Blocks (lbs) 2x tumber 1-1/8* Rim Board Plus 5,500 B,500 4,300 6,600

The construction details for residential designs are prone to changes.

Details released after April 2014 supersedes N-C301

Installation must comply with latest documentation on I-Joist and other Nordic products from the http://nordic.ca/

This document does not constitute a record of the structural integrity of the building nor suitability of the design assumptions made. Nordic Structures is responsible only for the structural adequacy of its component based on the design criteria and loadings shown on the calculation sheets.

(Nordic Request 1810-095)



N-C301/April 2014

MAXIMUM FLOOR SPANS

- . Maximum cleur spans applicable to single-span or multiple-span residential floor construction with a design live load of 40 year for all deal and of 15 pst. The ultimate limit states are based on the factored loads of 1.50.L + 1.250. The serviceshilly limit states include the consideration for floor vibration and at live load deflection limit of 1/480. For multiple-span applications, the end spans shall be 40% or married the adjacent span.
- or more at the adjacent span.

 2. Spann are beared on a composite floor with glued-native distinct strend beared (158th sheething with a minimum shitchess of 5% flow find for losts spenting of 19.2 inches or lest, or 3/4 such for folist spacing of 24 inches. Adherive shall meet the requirements from 1 CQBS-11,26. Standard, No concrete happing or bridging element was assumed, Increased spans may be achieved with the used of gypsum and/or a row of blocking at mid-span.
- . Minimum bearing length shall be 1-3/4 inches for the end bearings, and 3-1/2 inches for the intermediate bearings.
- Bearing stiffeners are not required when 1-joists are used with the spans and spacings given in this table, except as required for hungers.
- This span chert is based on uniform loads. For applications with other than uniform loads, an engineering analysis may be required based on the use of the design properties.
- Tables are based on Limit States Design per CAN/CSA O86-09 Standard, and NBC 2010.
- 7. Si units conversion: 1 inch = 25.4 mm 1 foot = 0.305 m

MAXIMUM FLOOR SPANS FOR NORDIC I-JOISTS SIMPLE AND MULTIPLE SPANS

	1000		Simple	spons			Muliiple	spans .	
Joist Depth	Joist Series	9.00	On centre	spacing			On confro	spacing	44.
		12"	16"	19.2	24"	12"	16"	19.2	24"
Sec. 35.55	Nt-20	15-1	14'-2"	13'-9"	13-5	16'-3*	15-4'	14'-10"	14'-7'
100	NI-40x	16-1*	15.2	14-8	14-9	17-5	16-5	15'-10"	15'-5'
9-1/2"	NI-60	16'-3"	15'-4"	14'-10"	14'-11'	17-7	16'-7"	16'-0"	16'-6"
1.00	N1-70	17:1"	16'-1"	15'-6"	15-7	18-7	17:4"	16'-9"	17-2
S. 1966.	NI-80	17'-3"	16-3	15'-8"	15-9	18-10	171.6	16'-11"	17-5
1000000	NI-20	16-11*	16'-0'	15'-5"	15-6"	18'-4"	17'-3"	16'-8'	16'-7"
1.00	NI-40x	18-1*	17'-0"	16'-5"	16'-6"	20-0	18.6	17'-9"	17-7
W. S. S.	N1-60	18'-4"	17.3	16.7*	16-9	20'-3"	18.9	18'0'	18'-9'
11-7/8	NI-70	19-6	18'-0"	17'-4"	17'-5"	21'-6"	19-11	19.0	19'-8'
400	NI 80	19'-9"	18'-3"	17-6*	17'-7"	21'-9"	20-2	19-3*	19-11*
10.300	NI-90	20-2*	18-7"	17-10	12-11*	22'-3'	20.7	19-8	19-9
100	NI-90x	20'-4"	18-9	17-11*	18'-0"	22.5	20.9	19-10	20-5
10.00	NI-40x	20'-1"	18-7	17'-10"	17:-11	22.2	20.6	19-8	19-4
	NI-60	20'-5"	18-11	18'-1"	18-2	22-7*	20-11-	20-0	20-10
14	NI-70	21'.7"	20.0	19-1	19-2	23-10	22-1	21-1	21'-10'
14	NI-80	21'-11"	20'3	19-4	19-5	24'-3"	22.5	21'-5"	22-2
3.73	NI-90	22-5	20'-8"	19-9	19-9'	24.9	22'-10"	21'-10"	21-10
35.70	NI-90x	22.7	20-11*	19-11-	20-0	25'-0"	23'-1"	22-0	22.9
72.5 33.5	NI-60	22-3	20.8	19-9	19-10*	24'-7"	22.9	21'-9'	22.9
	NI-70	23-6	21'-9"	20.9	20-10	26'-0"	24'-0"	22-11	23.9
16"	NI-80	23'-11"	22-1	21'-1"	21'-2"	26'-5'	24'-5"	23-3*	24-1
45 8 73	NI-90	24'-5'	22.6	21'-5"	21'-6'	26'-11"	24'-10"	23-9	23.9
14 Fe 30	NI.90x	24'-8"	22.9	21.9.	21,10,	27'-3"	25-2	24.0	24'-10"

1-JOIST HANGERS

- 2. All nailing must meet the hanger manufacturer's recommendations.
- Hangers should be selected based on the joist depth, flange width and load capacity based on the maximum spans.
- . Web stilleners are required when the sides of the hangers do not laterally brace the top flange of the 1-joist.





CCMC EVALUATION REPORT 13032-R

WEB STIFFENERS

RECOMMENDATIONS:

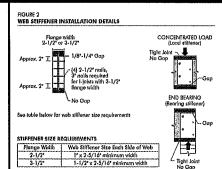
■ A bearing stiffener is required in all engineered applications with factored reactions greater than shown in the Hotel properties table found of the Hotel Construction Guide (C101). The gap between the stiffener and the flange is at the top.

A bearing stiffonor is required when the I-joist is supported in a hanger and the states of the hanger do not extend up to, and support, the top flange. The gap between the stiffener and flange is at the top.

sattener and flange is at the iop.

• A load stiffener is required at locations where a foctored concentrated load ground than 2,700 list is applied to the top flange between supports, or in the case of conditional conditions, or in the case of conditional conditions, anythere between the conditional conditions are supported by the condition of the condition of

Si units conversion: 1 inch = 25.4 mm



NORDIC I-JOIST SERIES 5-P-F No.2 1950FMSR 2100FMSR 1950FMSR 33 pieces 33 pieces per unit per unit 23 pleass per unit 23 pieces per unit

Chanilers Chibougomau Ltd. harvests its own trees, which enables Nordic products to adhere to strict quality control procadures throughout the manufacturing process. Every phase of the operation, from forest to the finished product, reflects our commisment to quality.

Nordic Engineered Wood I-joists use only linger-jointed black spruce lumber in their flonges, ensuring consistent quality, superior strength, and longer spon corrying capacity.

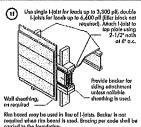


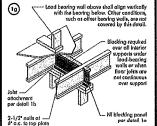
For nailing schedules for multiple beams, see the manufacturer's

Note: Unless hanger sides knorally support the top llange, bearing stiffeners shall be used.

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Nordic Lam or SCL

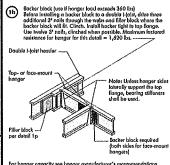




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l-joist per detail 1b

Tight Joint No Gap



BACKER BLOCKS (Blocks must be long enough to permit required nailing without splitting)

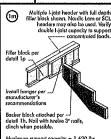
2-1/2" 1" 5-1/2"	Flange Width	Material Thickness Required*	Minimum Depth**		
	2-1/2*	1*	5-1/2"		
3-1/2* 1-1/2* 7-1/4*	3-1/2*	1-1/2*	7-1/4*		

Minimum grade for backer block material shall be S-F.F No. 2 or belter for solid saven lumber and wood structural panels conforming to CAN/CSA-0.437 Standard. For face-mount hangers use not joist depth minus 3-1/4" for joists with 1-1/2" thick flanges. For 2" thick flonges use net depth minus 4-1/4".



1/6" to 1/4" gap between top flange and filler block

®



⑽

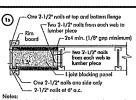
Maximum support capacity = 1,620 lbs

FILLER BLOCK REQUIREMENTS FOR DOUBLE I-JOIST CONSTRUCTION

℩

Note: Blocking required at bearing for lateral support, not shown for clarity.

Optional: Minimum 1x4 inch strap applied to underside of joist at blocking line or 1/2 inch minimum gypsum celling attached to underside of joists.



Notes:

In some local codes, blocking is prescriptively required in the first joint space (or first and second joint space) next to the starts joint. Where required, see local code requirement for spacing of the blocking.

All nails are common spiral in this detail.

- Support back of t-joist web during nailing to prevent damage to web/flange connection.
- Leave a 1/8 to 1/4-inch gap between top of filter block and bottom of top 1-joint
- for new books and solution to rep repair florage.

 Filler block is required between joists for foll length of span.

 Nati joists regarder with two rows of 3' onlise 10 greater on the control of 1' onlise 10 greater on the control of 1' onlise 10 greater of 1' onlise 10 greater of 1' onlise 1' on

3-1/2° x 1-1/2° 11-7/8 3-1/2° x

2-1/8" x 6" 2-1/6" x 8" 2-1/8" x 10" 2-1/8" x 12"

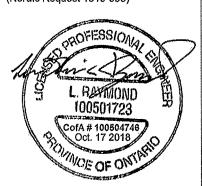
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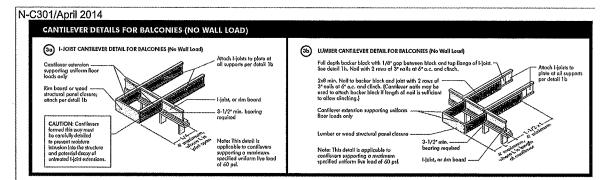
Details released after April 2014 supersedes N-C301

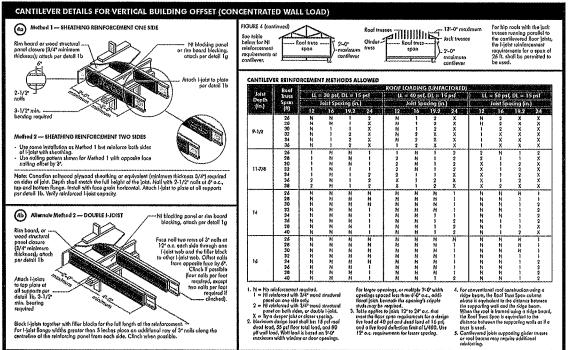
Installation must comply with latest documentation on I-Joist and other Nordic products from the http://nordic.ca/

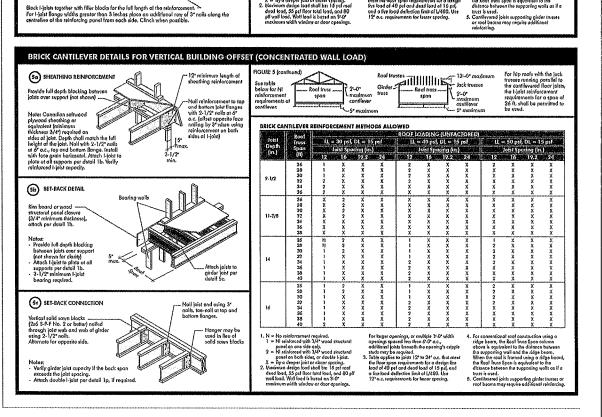
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(Nordic Request 1810-095)









The construction details for residential designs are prone to changes.

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(Nordic Request 1810-095)



N-C301/April 2014

WEB HOLES

RULES FOR CUTTING HOLES AND DUCT CHASE OPENINGS:

- The distance between the inside edge of the support and the centreline of any hale or duct chase opening shall be in compliance with the requirements of Table 1 or 2, respectively.

 I-joint top and bottom flanges must NEVER be out, notched, or otherwise modified.
- 3. Whenever possible, field-cut holes should be centred on the middle of the web.
- Triburral (possible) amount failes strong to be tentined on the miscine of the vector. The maximum stap halo or the maximum depth of a duct chase populing that can be cut into an i-joist was stall equal the clear distance between the flanges of the i-joist minus 1/4 inch. A relatinum of 1/8 Inch, should always be maintained between the top or bottom of the halo or opening and the adjacent i-joist flange.
- The sides of square holes or longest sides of rectangular holes should not exceed 3/4 of the diameter of the maximum round hole permitted at that location.
- 3/4 of the diameter of the maximum round hole permitted at that facation.

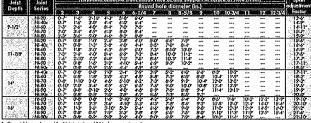
 4. Where rans a thom one hale is recessary, the distance a brusen edigicant hole edges shall exceed twice the diameter of the largest round hale or twice the size of the largest aware hale for rivice tile largest rectangular hale or dust clisse opening and each hole and duct chare opening that the sized and becated in compliance with the requirements of Tables 1 and 2, respectively.

 A kineckost is not considered a hole, may be utilized anywhere it occurs, and may be ignored for purposes of colcularing minimum distances between holes and/or duct chare openings.

 3. Holes recording 1-1/2 hackes or smaller shall be parallelad onywhere in a conflictivated action of a joist. Holes of groofer size may be apprentited subject to verification.

- A 1-1/2 inch hole or smaller can be placed anywhere in the web provided that it
 meets the regularments of rule number 6 above.
- 10. All holes and duct chase openings shall be cut in a workman-like manner in accordance with the restrictions listed above and as illustrated in Figure 7.
- 11. Limit three maximum size holes per span, of which one may be a duct chase
- A group of round holes at approximately the same location shall be permitted if they meet the requirements for a single round halo circumscribed around them.

TABLE 1 LOCATION OF CIRCULAR HOLES IN JOIST WEBS Simple or Multiple Span for Dead Louds up to 15 psf and Live Loads up to 40 psf



Above table may be used for Lipids spacing of 24 inches an earlier or less. Holy location distance is measured from inside loce of supports to centre of licke Distances in this chart are based on uniformly looded joists.

The above table is based on the 1-joint used at their maximum span. If the 1-joints are placed at less than their full maximum span (see Maximum Floor Spans), the minimum distance from the centralized the late face of any support (D) as given above may be reduced as follows:

Oreduced in Sape In Dreduced in Oreduced in Distance from the inside lose of any appart to centre of hole, reduced for less shart maximum span applications (II). The reduced distance shall not be less than in subset from the late of the support to edge of the hole. The new part of the centre of the support of the subset in the subset in the subset in the subset of support (II). Span Adjustion Facility given in this table.

Span Adjustion Facility given in this table.

The meritum distance from the well-due of any support to centre of their from this table.

If agreed in genetic from 1, vice 1 in the observe coloutation for agreed.

FIGURE 7 FIELD-CUT HOLE LOCATOR

A knockout is NOT considered a hole, may be utilized wherever it occurs and may be ignored for purposes of calculating minimum distances between holes.



For reatingular holes, avoid over-cutting the comers, as this can couse unnessess stress concentrations. Slighth rounding the comers is recommended. Starting the comers is recommended. Starting the reatingular hole by diffilling a 1-Inch diamater hole in each of the four correr and them notificing the case between the holes is another good reathed to minimize damage to the 1-jobs.



com hibb may be used for hight spocing of 24 inches on centre of law.

The characterising location destores in measured from hidde loca of sepocits to centre of opening, as done which is broad on simple-upon points only. To other applications, control your local distributor, allower are located your local distributor, allower are located in undermy located local youth his memory for each requiremental for a design has located (A.D. put and and also all 18 put and a her book differents from the U.D.C.) for other applications, contact your local distributor.

INSTALLING THE GLUED FLOOR SYSTEM

- 1. Yips any mud, dirt, water, or ice from I-joint flanges before gluing.
- 2. Snap a chalk line across the t-joists four feet in from the wall for panel edge alignment and as a boundary for spreading glue.
- Spread only onough give to key one or two panels at a lime, or follow specific recommendations from the give manufacturer.
- Luy the first panel with tongue side to the wall, and noil in place. This protects the tongue of the next
 panel from damage when tapped into place with a block and sledgehammer.
- Apply a continuous line of glue (about 1/A-inch diarneter) to the top flange of a single I-joist. Apply glue in a winding pattern on wide areas, such as with double I-joists.
- 6. Apply two lines of give on Holass where panel ends but to assure proper gluing of each end.
 7. Altar the first row of panels is in place, spread give in the groove of one or two panels at a time before bying the east row. Often the marry to continuous or spaced, but good squeeze-out by applying at himmer line (10) linel) then used an Holass longue.
- 8. Tap the second row of panels into place, using a block to protect groove edges.
- Stagger and joints in each succeeding row of panels. A 1/8-inch space between all end joints and 1/8 inch at 03 edges, including 18G9 edges, is recommended. (Use a specar tool or an 2-1/2" comm notil to surve accesses and constraint specing.)
- not to assure accurate and combitant spacing.)

 10. Compilate all nating of each panel before give sets. Check the manufacturer's recommendations for awe time. (Warm weather accelerates give setting.) Use 2' ring- or setter-shank rolls for panels 3/4-thet hibits or bass, and 2-1/2' ring- or setter-shank rolls for thickey ponols. Space notils per the table below. (Cases and Beparking may be required by some codes, or for disphagen construction. The flishind deck can be walked on right away and will carry construction loads without damage to the gibb band.

fasteners for sheathing and subflooring(1)

Maximum	Minimum		il Size and Ty	Maximum Spacing			
Joist Spaking (in.)	. Panel Thickness (in.)	Common Wire or Spiral Nails	King Thread Nais or Screws	Skaples	of Fas Edges	ferters Inform Supports	
16	5/8	2'	1-3/4*	2'	6'	12'	
20	5/8	2'	1-3/4*	2'	6*	12'	
24	3/4	2'	1-3/4"	2"	6'	12'	

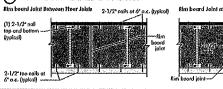
- 1. Fasteners of sheathing and subflooring shall conform to the above table.
- 2. Staples shall not be less than 1/16-inch in diarneter or thickness, with not less than a 3/8-inch crown driven with the crown parallel to faunting.
- 3. Flooring screys shall not be lass than 1/8-inch in diameter
- Special conditions may impose heavy traffic and concentrated loads that require construction in excess
 of the minimums shown.
- 5. Use only adherives conforming to CAN/COSE-71.26 Standard, Adherives for Field-Gluing Plywood to Lumber Framing for Floor System, applied in accordance with the manufacturer's recommendations. If OSB panels with socied surfaces and edges are to be used, use only solvent-based gives; check with panel manufactures.

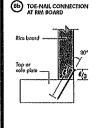
Ref.: NRC-CNRC, National Building Code of Canada 2010, Table 9.23.3.5.

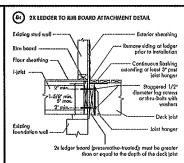
IMPORTANT NOTE:

Floor shouthing must be field glued to the I-joist flunges in order to achieve the maximum spans shown in this document. If sheathing is nailed only, I-joist spans must be verified with vary local distributor.

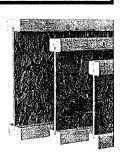
RIM BOARD INSTALLATION DETAILS (8) ATTACHMENT DETAILS WHERE RIM BOARDS ABUT











1-1/2"

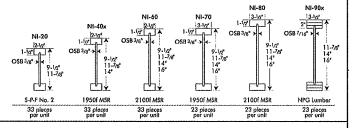
CONSTRUCTION DETAILS FOR RESIDENTIAL FLOORS



www.nordicewp.com

Refer to the Installation Guide for Residential Floors for additional information.





WEB HOLE SPECIFICATIONS

CCMC EVALUATION REPORT 13032-R

RULES FOR CUTTING HOLES AND DUCT CHASE OPENINGS:

- The distance between the inside edge of the support and the controlline of any hole or duct chase opening shall be in compliance with the requirements of Tablet are 2, respectively.
 Helds to go and bettom langers must NEVER be cut, notched, or otherwise modified.
 Whenever possible, field-cut holes should be centred on the middle of the web.
 He make minum size hole or the maximum depth of a duct chase opaning that can be cut into an Helds when shall equal the clear distance between the flanges of the Helds into 114 lack. A mininum of 118 lack hall dawys be maintained between the top or bottom of the hole or opening and the adjacent Helds flange.
- 5. The sides of square holes or langest sides of rectangular holes should not exceed 3/4 of the diameter of the maximum round hole permitted at that location.
 6. Where more than one hole is necessary, the distance between adjacent hole edges shall acceed hive the diameter of the largest round hole or threa the size of the largest square hole for hive the file of the largest square hole for hive the largest hole or duct chave openingly and each hole and duct chave opening shall be sized and located in compliance with the requirements of Tables 1 and 2, respectively.
 7. A knockout is not considered a hole, may be vilized anywhere it occurs, and may be ignored for purposes of calculating minimum distances between holes and/or duct chase openings.
 8. Holes moesavring 1-1/2 Inches or smaller are permitted anywhere in a cantilevered section of a joist. Holes of greater size may be permitted subject to verification.

N-C303 / September 2013

- 9. A 1-1/2 Inch hole or smaller can be placed anywhere in the web provided that it meets the requirements of rule number 6 above.
 10. All holes and duct chase openings shall be cut in a overkmet with the restrictions listed above and as illustrated in Figure 7.

 11. Limit shree maximum size holes per span, of which one may be a duct chose opening.

 12. A group of round holes of approximately the same location shall be permitted if they meet the requirements for a single round hole accumscribed around titem.

LOCATION OF CIRCULAR HOLES IN JOIST WEBS

Simple or Multiple Span for Dead Loads up to 15 psf and Live Loads up to 40 psf

Joist Joist Depth Series		Minimum Distance from Inside Face of Any Support to Centre of Hale (ft - in.)														
	Round Hole Diameter (in.)															
	201103	2	3	4	5	6	6-1/4	7	8	8-5/8	9	10	10-3/4	11	12	12-3/4
	NI-20	0'-7*	1'-6"	2'-10"	4'-3'	5'-8'	6'-0"		*	***			***	***		***
9-1/21	NI-40x	0:-7"	1'-6"	3'-0"	4'-4"	6'-0'	6'-4"	***	***	***			***	***	***	***
7-174	NI-60	1'-3'	2'-6"	4'-0"	5'-4"	7'-0'	7'-5"	***	***	***	***	***	***	***		***
	NI-70	2:0	3'-4"	4'-9*	6'-3"	8'-0"	8'-4"	***	***	***	***	***	***	***		***
	MI-80	2'-3'	3'-6"	5'-0"	6'-6"	8'-2"	8'-8'	***	***	***	***	***	-44	***		44.
	NI-20	0.7	0'-8"	1'0'	2'-4"	3'-8"	4'-0"	5'-0"	6'-6"	7'-9"		***	74>	***	***	***
	NI-40x	0'-7"	0'-8"	1'-3"	2'-8'	4'-0"	4-4	5'-5"	7'-0"	8'-4"		***	***	***	***	***
11-7/8	NI-60	0'-7"	1'-8"	3'-0"	4'-3"	5.9	6'-0"	7'-3"	8'-10"	10'-0"	***	***	***	***	***	***
	NI-70	1'-3"	2'-6"	4'-0"	5'-4"	6.9	7'-2"	8'-4"	10'-0"	11'-2"	***	***	***	***	***	***
	NI-80	1.6	2'-10"	4'-2"	5'-6"	7'-0'	7'-5'	8'-6"	10'-3"	11'-4"		***	***	***	***	***
	NI-90x	0'-7'	0.8.	0'-9"	2'-5'	4'-4"	4'-9"	6'-3"	***		***	***	***	***	~**	***
	NI-40x	0.7	0.84	0'-8"	1'-0"	2'-4"	2'-9"	3'-9"	5'-2"	6'-0"	6'-6"	8'-3"	10'-2"	***	***	P44
14"	NI-60	0.7	0.84	14.81	3'-0"	4'-3'	4'+8"	5'-8'	7'-2"	8'-0"	8'-8"	10.4	11:9"	***	***	F#4
14	NI-70	0.8	1'-10"	3'-0"	4-5	5'-10"	6'-2"	7'-3°	8'-9"	9'-9"	10'-4"	12'-0'	13'-5"	***	***	***
	NI-80	0.10	2'-0'	3'-4"	4-9	6.2	6'-5"	7'-6"	9'-0"	10'-0"	10'-8'	12'-4"	13'-9"	***	***	***
	NI-90x	0'-7"	0'-8'	0'-81	2'-0'	3'-9'	4'-2"	5'-5"	7'-3"	8'-5"	9'-2"		***	***		***
16'	MI-60	0.7'	0'-8"	0'-8"	1'-6"	2'-10'		41.2"	5'-6"	6'-4"	7'-0"	8'-5"	9'-8'	10-2	12'-2"	13'+9'
10.	NI-70	0-7"	1'-0"	2'-3"	3'-6"	4'-10'		6'-3"	7'-8"	8'-6"	9'-2"	10'-8'	12'-0"		14'-0"	15'-6"
	NI-80	0.7"	14-3"	2-6°	3'-10'	5'-3'	5'-6"	6'-6"	8'-0"	9'-0'	9'-5"	11'-0"	12'-3"	12'-9'	14'-5"	16'-0'
	NI-90x	0.7*	0'-8"	0.9	2'-0'	3'-6"	4'-0'	5'-0"	61.91	7'-9"	8'-4"	10'-2"	11'-6"	12'-0"	***	***

- 1. Above table may be used for 1-joint spacing of 24 inches on centra or less.
 2. Hole location distance is measured from inside face of supports to centre of hole.
 3. Distances in this chart are based on uniformly loaded joists.
 4. The above stable is based on the 1-joints bearing used at their maximum spans. The minimum distance as given above may be reduced for shorter spans; contact your local distributor.

DUCT CHASE OPENING SIZES AND LOCATIONS Simple Span Only

Joist Depth	Joist Series	Minimum Distance from Inside Face of Supports to Centre of Opening (ft - in.)										
		Duct Chase Length (in.)										
		8	10	12	14	16	18	20	22	24		
9-1/2*	NI-20 NI-40x NI-60 NI-70 NI-80	4'-1" 5'-3" 5'-4" 5'-1" 5'-3"	4'-5' 5'-8' 5'-9' 5'-5' 5'-8'	4'-10' 6'-0' 6'-2' 5'-10' 6'-0'	5'-4" 6'-5" 6'-7" 6'-3" 6'-5"	5'-8" 6'-10' 7'-1" 6'-7" 6'-10'	6'-1" 7'-3" 7'-5" 7'-1" 7'-3"	6'-6" 7'-8" 8'-0" 7'-6" 7'-8"	7'-1" 8'-2" 8'-3" 8'-1" 8'-2"	7'-5' 8'-6' 8'-9' 8'-4' 8'-6'		
11-7/8*	NI-20 NI-40x NI-60 NI-70 NI-80 NI-90x	5-9' 6-8' 7-3' 7-1' 7-2' 7-7'	6'-2' 7'-2' 7'-8' 7'-4' 7'-7' 8'-1'	6'-6' 7'-6' 8'-0' 7'-9' 8'-0' 8'-5'	7'-1" 8'-1" 8'-6" 8'-6" 8'-5" 8'-5"	7'-5" 8'-6" 9'-0" 8'-7" 8'-10" 9'-4"	7'-9" 9'-1" 9'-3" 9'-3" 9'-3"	8'-3" 9'-6" 9'-9" 9'-6" 9'-8" 10'-2"	8'-9" 10'-1" 10'-3" 10'-1" 10'-2" 10'-8"	9'-4" 10'-9' 11'-0' 10'-4' 10'-8' 11'-2'		
14°	NI-40x NI-60 NI-70 NI-80 NI-90x	8'-1* 8'-9' 8'-7' 9'-0' 9'-4'	8:-7' 9:-3' 9:-1' 9:-3'	9'-0' 9'-8' 9'-5' 10'-3'	9'-6" 10'-1" 9'-10" 10'-1" 10'-7"	10'-1" 10'-6" 10'-4" 10'-7"	10'-7' 11'-1' 10'-8' 11'-1'	11'-2' 11'-6' 11'-2' 11'-6' 12'-1'	12'-0' 13'-3' 11'-7' 12'-1' 12'-7'	12'-8' 13'-0' 12'-3' 12'-6' 13'-2'		
16"	NI-60 NI-70 NI-80 NI-90x	10'-3' 10'-1' 10'-4' 11'-1'	10-8 10-5 10-9 11-5	11'-2' 11'-0' 11'-3' 11'-10'	11'-6" 11'-4" 11'-9" 12'-4"	12'-1° 11'-10' 12'-1° 12'-10'	12'-6' 12'-3' 12'-7' 13'-2'	13'-2" 12'-8" 13'-1" 13'-9"	14-1" 13-3" 13-8" 14-4"	14'-10' 14'-0' 14'-4' 15'-2'		

- Above table may be used for I-joist spacing of 24 inches an centre or less.
 Duct chase opening location distance is measured from Inside face of supports to centre of opening.
 The above table is based on simple-span joists only. For other applications, contact your local distributor.
 Distances are based on uniformly loaded floor joists that meet the span requirements for a design live load of 40 ps and deed load of 15 pst, and a live load delication limit of I/480.
 The obove table is based on the I-joist being used at their maximum spans. The minimum distance as given above may be reduced for shorter spans; contact your local distributor.

FIGURE 7

FIELD-CUT HOLE LOCATOR

Duct chase opening (see Table 2 for minimum distance from bearing) 2x duct chase length or hole diameter. 2x diameter of larger hole or hole diamore, whichever is larger Maintain minimum 1/8" space between top and bottom flange --- all duct chose openings and holes



Knockouts are prescored holes provided for the contractor's convenience to install electrical or small plumbing lines. They are 1-1/2 inches in diameter, and are spaced 15 linches on canira along the length of the I-plats. Where possible, it is preferable to use knockouts instead of field-cut holes.

Never drill, cut or notch the flange, or over-cut the vieb.

Holes in webs should be cut with a sharp sow

For rectongular hales, avoid over-cutting the corners, as this can cause unnecessary stress concentrations. Slightly rounding the corners is recommended. Starling the rectongular hole by drilling a 1-inch diameter hole in each of the four corners and then naking the cuts between the holes is another good method to minimize damage to the 1-joist.

SAFETY AND CONSTRUCTION PRECAUTIONS



WARNING: I-joists are not stable until completely installed, and will not carry any load until fully braced and sheathed. AVOID ACCIDENTS BY FOLLOWING THESE IMPORTANT GUIDELINES:

- AVOID ACCIDENTS BY FOLLOWING THESE MAPORTANT GUIDELINES:

 I Frace and nail each I-joist or is its installed, usign panages, blocking panels, tim board, and/or cross-bridging at joist ends. When I-joists are applied continuous over interior supports and a load-bearing wall is planned at that location, blocking will be required at the interior support.

 2. When the building is completed, the floor shealthing will provide lateral support for the top flonges of the I-joists. Until this shealthing is papiled, emporary bracting, often called situs, or temporary shealthing must be applied to prevent I-joist rollover or buckling.

 I Temporary bracing or struts must be 1x4 inch minimum, at least 8 feet long and spaced no more than 8 feet an centre, and must be secured with a minimum of two 2-1/2² noils fastened to the top surface of each I-joist. Noil the bracing to a lateral restraint at the end of each box, to pends of adjoining bracing over a least two I-joists, and it is not a lateral restraint at the end of each box, to pends of adjoining bracing over a least two I-joists at the end of the box.

 3. For contilevered I-joists, received por and bottom flanges, and those ends with closure panels, rim board, or cross-bridging.

 4. Install and fully nail permanent shealthing to each I-joist before placing loads on the floor system. Then, stack building moterials over beams or wolls only.

 5. Never install a damaged I-joist.

Improper storage or installation, failure to follow applicable building codes, failure to follow span ratings for Nordic I-joists, failure to follow allowable hale sizes and locations, or failure to use web stiffaners when required can result in serious ocadents, Follow these installation guiddlines carefully.



PRODUCT WARRANTY

Chantiers Chibongaman guarantees that, in accordance with our specifications, Nordic products are free from manufacturing defects in material and workmanship.

Furthermore, Chantiers Chibongaman warrants that our products, then militeed in accordance with our bandling and installation instructions, will meet or exceed our specifications for the lifetime of the structure.



The construction details for residential designs are prone to changes.

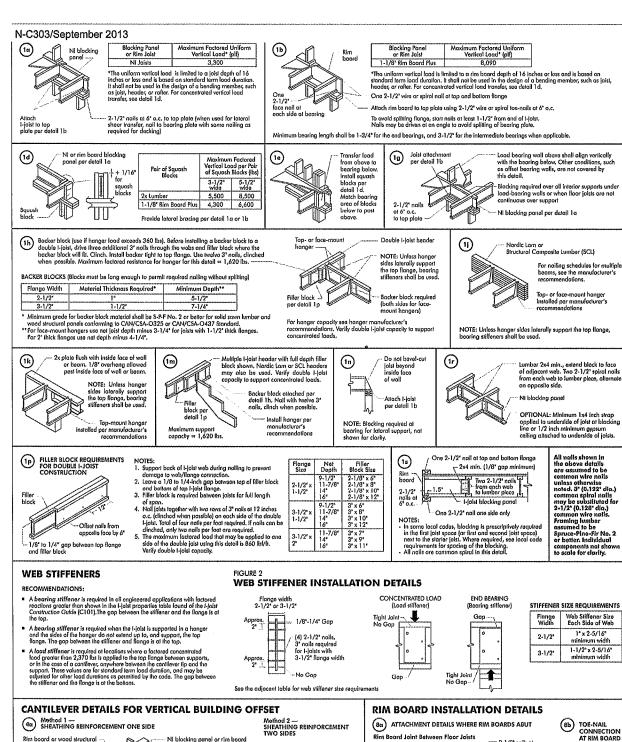
Details released after September 2013 supersedes N-303

Installation must comply with latest documentation on I-Joist and other Nordic products from the http://nordic.ca/

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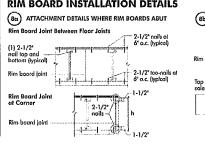


(Nordic Request 1810-095)



Method 2 — SHEATHING REINFORCEMENT TWO SIDES 46 SHEATHING REINFORCEMENT ONE SIDE Rim board or wood structural panel closure (3/4* minimum thickness); altach per detail 1b NI blocking panel or rim board blocking, atlach per detail 1g Uso same installation as Method 1 but reinforce both sides of I-joist with sheathing. Allach I-joist to plate per detail 1h pattern shows for Method 1 with opposite face nailing offset by 3*. 2-1/2' nails 3-1/2" min. bearing required

NOTE: Canadian softwood plywood shealthing or equivalent (minimum thickness 3/4") required on sides of joist. Depth shall match the full height of the joist. Natil with 2-1/2" acits at 6" a.c., top and bottom flange, install with face grain horizontal. Attack-light to plate of all supports per detail 15. Natil viri prindroad-light expectity.



30° *t*/3



The construction details for residential designs are prone to changes.

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