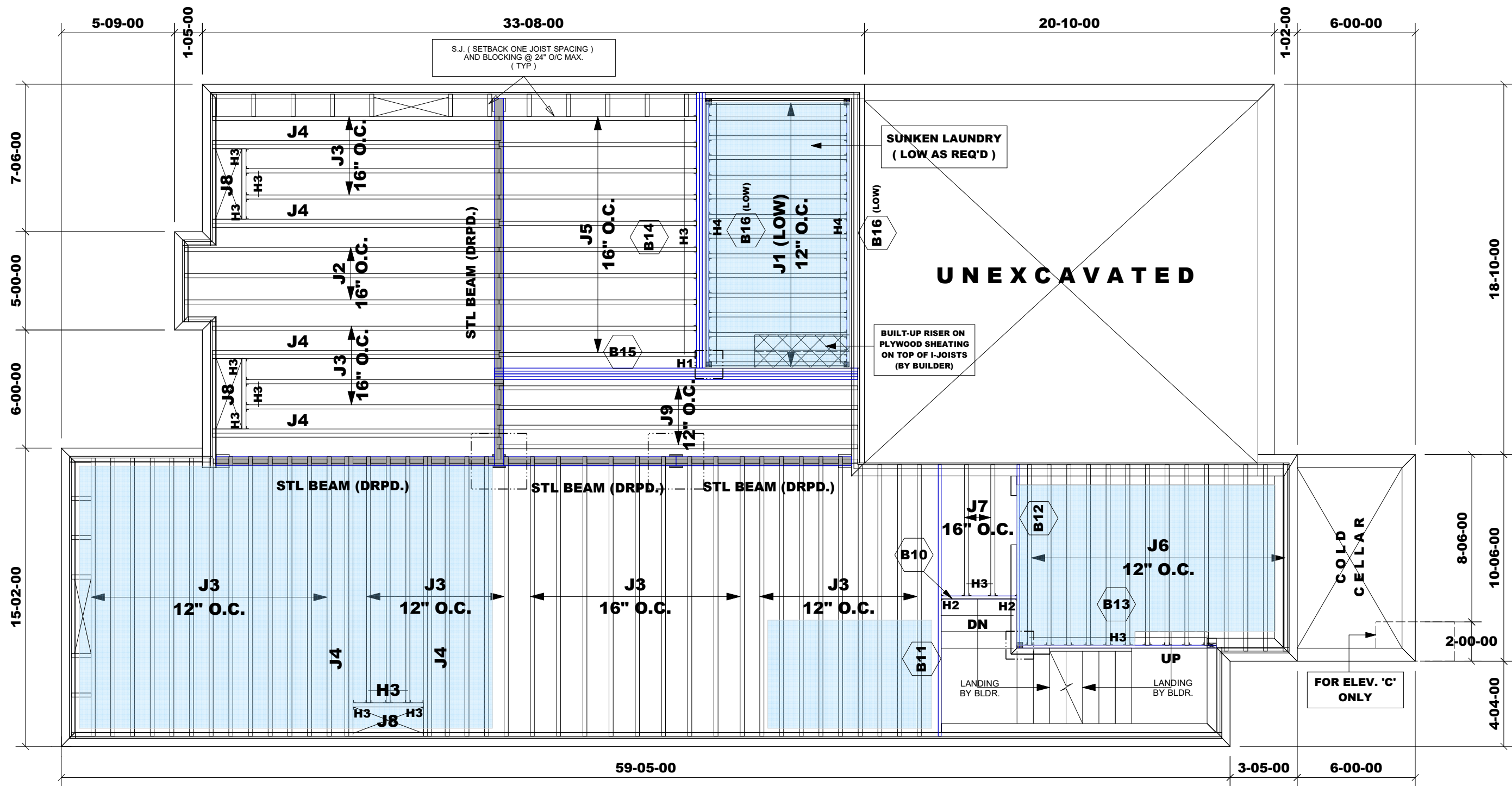




FIRST FLOOR FRAMING

UNIT : 4205 (BROADVIEW)
ELEV. " A, B & C "

Do not scale - refer to architectural
plans for dimensions

HATCH LEGEND	
	CERAMIC TILES

Products				
PlotID	Length	Product	Plies	Net Qty
B10	4-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
B11	14-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
B12	10-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
B13	10-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
B14	15-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	3	3
B15	19-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	4	4
B16	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	2
J1	8-00-00	9 1/2" NI-20	1	15
J2	16-00-00	11 7/8" NI-20	1	3
J3	15-00-00	11 7/8" NI-20	1	44
J4	15-00-00	11 7/8" NI-20	2	12
J5	11-00-00	11 7/8" NI-20	1	10
J6	10-00-00	11 7/8" NI-20	1	14
J7	7-00-00	11 7/8" NI-20	1	2
J8	4-00-00	11 7/8" NI-20	1	3
J9	19-00-00	11 7/8" NI-40x	1	4
xBk1	60-00-00	11 7/8" NI-20	1	1
xCa1	12-00-00	1 1/8" x 9 1/2" Rim Board	1	2
xCa2	12-00-00	1 1/8" x 11 7/8" Rim Board	1	16

Blocking panels are required over all interior supports under
load bearing walls or when floor joists are not continuous over
support.

Squash blocks are required under concentrated load.

Ceramic tile application as per O.B.C. 9.30.6

SUBFLOOR - 3/4" NAILED & GLUED *

RIMBOARD
1 - 1-1/8" X 11 7/8" O.S.B.

Connector Summary			
PlotID	Qty	Manuf	Product
H1	1		HGUS5.50/10
H2	2		HUS1.81/10
H3	35		LT251188
H4	30		LT259

SE020574 - SE020594

PL:109955



Yard:
Home Lumber

Contact:
Derek Frankfort

Date: June 20, 2020

ALPA ROOF
TRUSSES INC.

Gold Park

4205 (Broadview)

Pine Valley

LOCATION:
Vaughan

DESIGNER : FC

REVISIONS:

1	20210121
2	#2
3	#3
4	#4

LEGEND:

BBO	Beam By Others
APP	As Per Plan
PL(G)	Point Load from Girder
PL(W)	Point Load from Lintel
S.B.	Solid Bearing

DESIGN INFO:

JT/PL:

45147/106927

Layout ID:

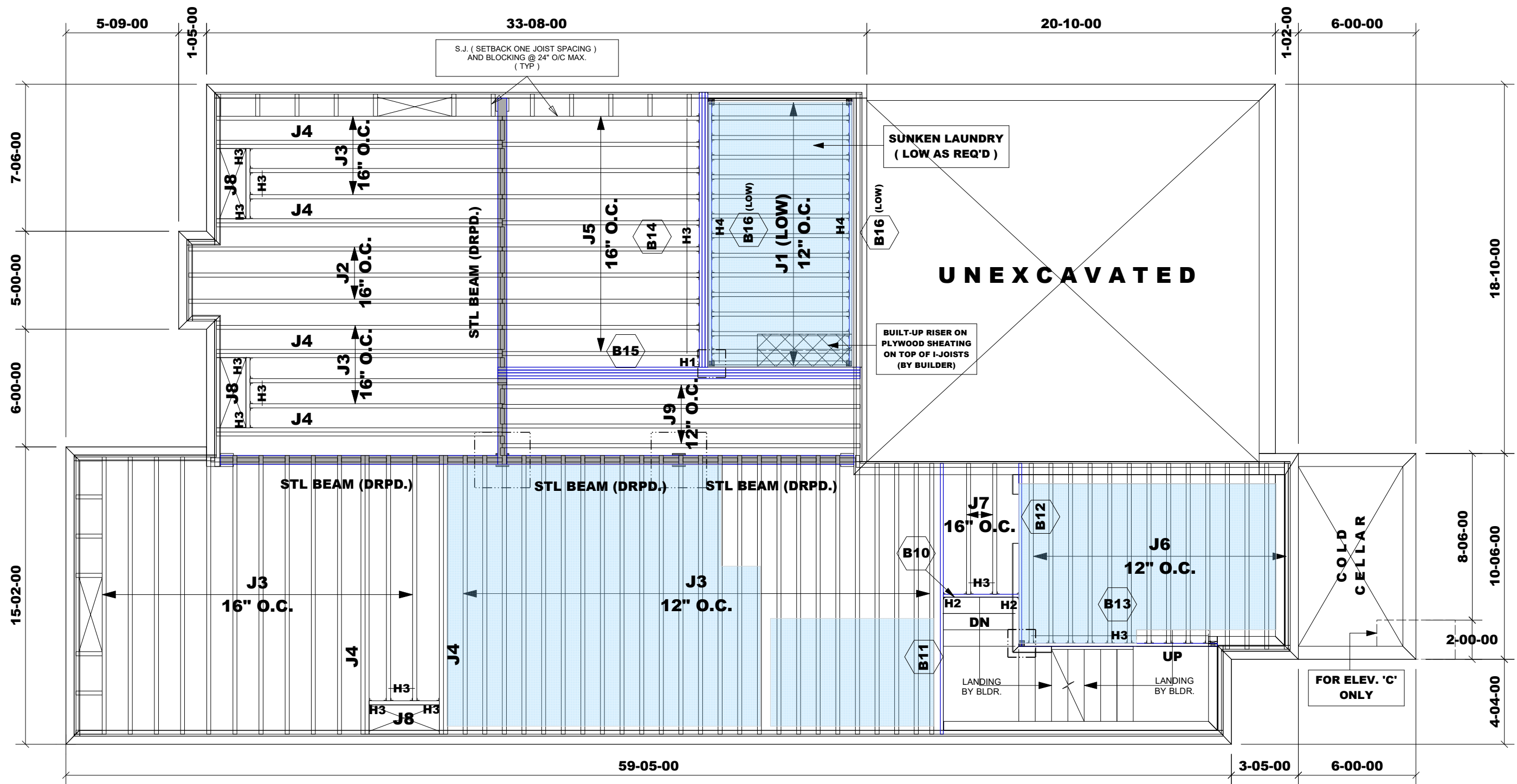
(321527)328366

LOADING

Live:	40.0 lb/ft²
Dead:	20.0 lb/ft²

DEFLECTION CRITERIA

Live:	360.000
Total	240.000



FIRST FLOOR FRAMING

UNIT : 4205 (BROADVIEW)
ELEV. " A, B & C " (OPTION)

Do not scale - refer to architectural
plans for dimensions

HATCH LEGEND	
	CERAMIC TILES

Products				
PlotID	Length	Product	Plies	Net Qty
B10	4-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
B11	14-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
B12	10-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
B13	10-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
B14	15-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	3	3
B15	19-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	4	4
B16	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	2
J1	8-00-00	9 1/2" NI-20	1	15
J2	16-00-00	11 7/8" NI-20	1	3
J3	15-00-00	11 7/8" NI-20	1	43
J4	15-00-00	11 7/8" NI-20	2	12
J5	11-00-00	11 7/8" NI-20	1	10
J6	10-00-00	11 7/8" NI-20	1	14
J7	7-00-00	11 7/8" NI-20	1	2
J8	4-00-00	11 7/8" NI-20	1	3
J9	19-00-00	11 7/8" NI-40x	1	4
xBk1	62-00-00	11 7/8" NI-20	1	1
xCa1	12-00-00	1 1/8" x 9 1/2" Rim Board	1	2
xCa2	12-00-00	1 1/8" x 11 7/8" Rim Board	1	16

Blocking panels are required over all interior supports under
load bearing walls or when floor joists are not continuous over
support.

Squash blocks are required under concentrated load.

Ceramic tile application as per O.B.C. 9.30.6

SUBFLOOR - 3/4" NAILED & GLUED *

RIMBOARD
1 - 1-1/8" X 11 7/8" O.S.B.

Connector Summary			
PlotID	Qty	Manuf	Product
H1	1		HGUS5.50/10
H2	2		HUS1.81/10
H3	34		LT251188
H4	30		LT259

PL:109955



Yard:
Home Lumber

Contact:
Derek Frankfort

Date: June 20, 2020

ALPA ROOF
TRUSSES INC.

Gold Park

4205 (Broadview)

Pine Valley

LOCATION:
Vaughan

DESIGNER : FC

REVISIONS:

1	20210121
2	#2
3	#3
4	#4

LEGEND:

BBO	Beam By Others
APP	As Per Plan
PL(G)	Point Load from Girder
PL(W)	Point Load from Lintel
S.B.	Solid Bearing

DESIGN INFO:

JT/PL:

45147/106927

Layout ID:

(321527)328366

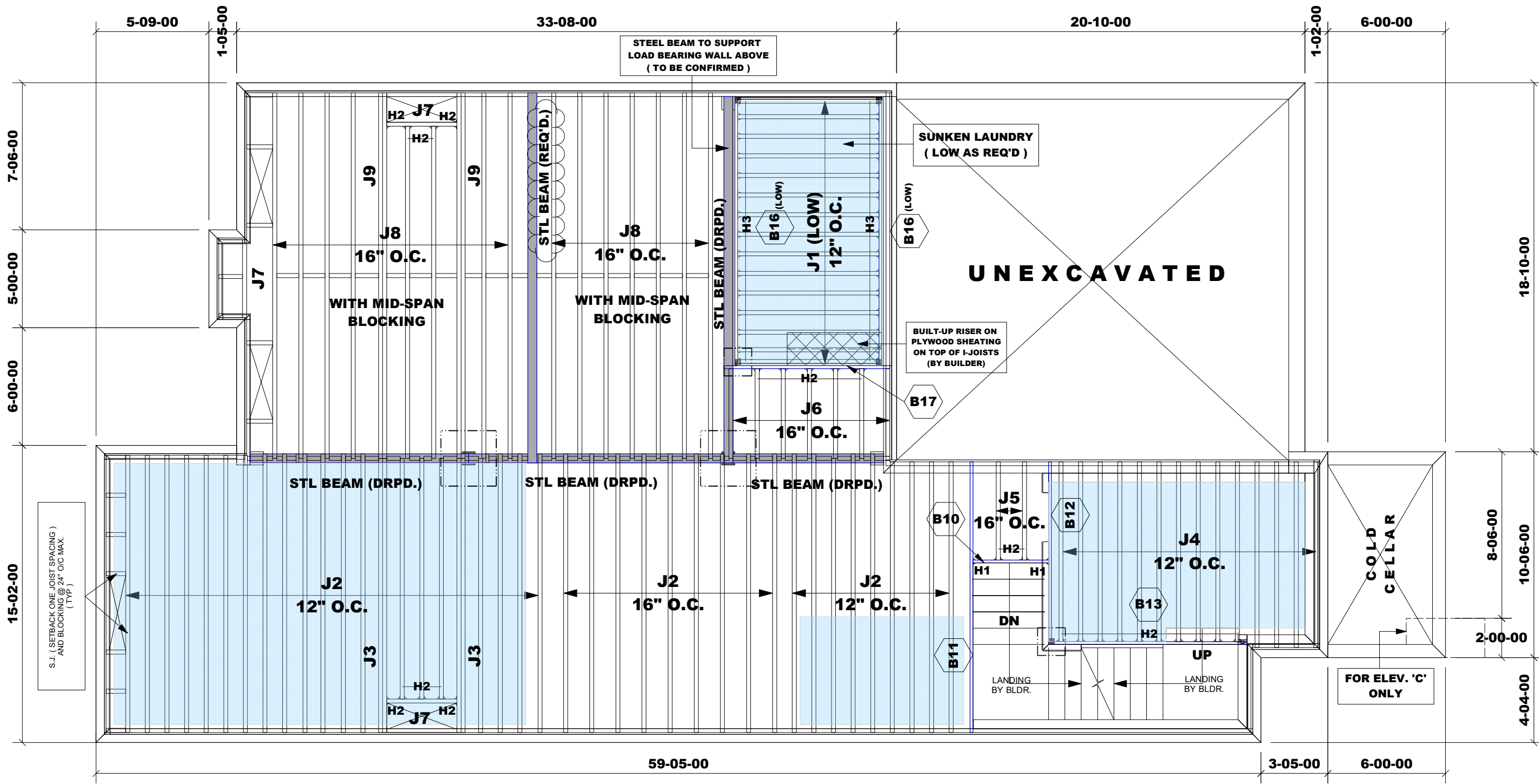
LOADING

Live:	40.0 lb/ft²
Dead:	20.0 lb/ft²

DEFLECTION CRITERIA

Live:	360.000
Total	240.000

2 OF 10



FIRST FLOOR FRAMING

UNIT : 4205 (BROADVIEW)
ELEV. " A, B & C "
OPT. 9' BASEMENT

Do not scale - refer to architectural
plans for dimensions

HATCH LEGEND	
	CERAMIC TILES

Products				
PlotID	Length	Product	Plies	Net Qty
B10	4-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
B11	14-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
B12	10-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
B13	10-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
B16	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	2
B17	9-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
J1	8-00-00	9 1/2" NI-20	1	15
J2	15-00-00	11 7/8" NI-20	1	38
J3	14-00-00	11 7/8" NI-20	2	4
J4	10-00-00	11 7/8" NI-20	1	14
J5	6-00-00	11 7/8" NI-20	1	2
J6	5-00-00	11 7/8" NI-20	1	7
J7	4-00-00	11 7/8" NI-20	1	3
J8	19-00-00	11 7/8" NI-40x	1	15
J9	19-00-00	11 7/8" NI-40x	2	4
xBk1	57-00-00	11 7/8" NI-20	1	1
xCa1	12-00-00	1 1/8" x 9 1/2" Rim Board	1	2
xCa2	12-00-00	1 1/8" x 11 7/8" Rim Board	1	17

Blocking panels are required over all interior supports under
load bearing walls or when floor joists are not continuous over
support.

Squash blocks are required under concentrated load.

Ceramic tile application as per O.B.C. 9.30.6

SUBFLOOR - 3/4" NAILED & GLUED *

RIMBOARD
1 - 1-1/8" X 11 7/8" O.S.B.

Connector Summary			
PlotID	Qty	Manuf	Product
H1	2		HUS1.81/10
H2	26		LT251188
H3	30		LT259

PL:109955



Yard:
Home Lumber

Contact:
Derek Frankfort

Date: June 20, 2020

ALPA ROOF
TRUSSES INC.

Gold Park

4205 (Broadview)

Pine Valley

LOCATION:
Vaughan

DESIGNER : FC

REVISIONS:

1	20210121
2	#2
3	#3
4	#4

LEGEND:

BBO	Beam By Others
APP	As Per Plan
PL(G)	Point Load from Girder
PL(W)	Point Load from Lintel
S.B.	Solid Bearing

DESIGN INFO:

JT/PL:

45147/106927

Layout ID:

(321527)328366

LOADING

Live:	40.0 lb/ft²
Dead:	20.0 lb/ft²

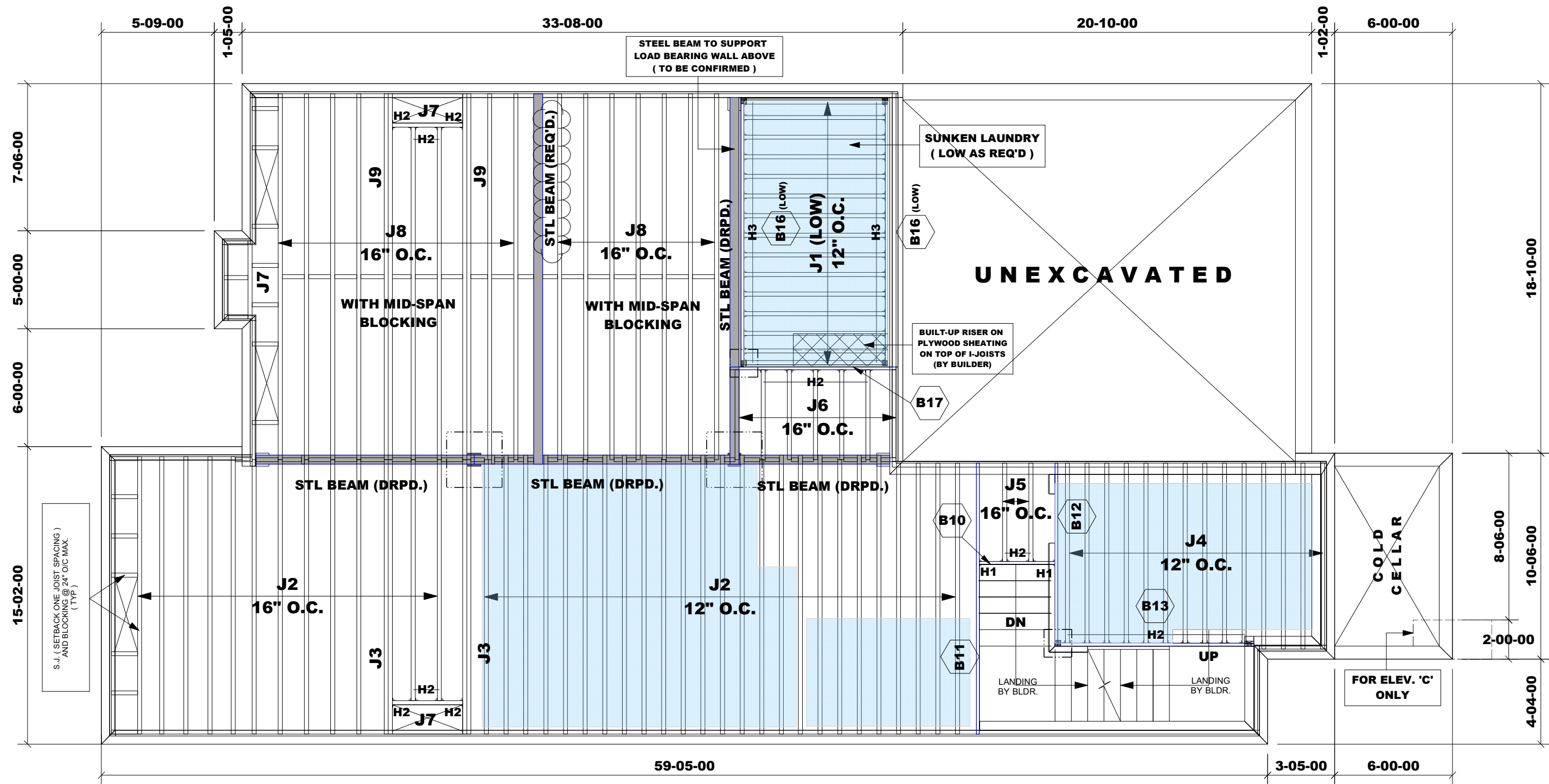
DEFLECTION CRITERIA

Live:	360.000
Total	240.000

3

OF

10



FIRST FLOOR FRAMING

UNIT : 4205 (BROADVIEW)
ELEV. " A, B & C " (OPTION)
OPT. 9' BASEMENT

Do not scale - refer to architectural
plans for dimensions

HATCH LEGEND	
	CERAMIC TILES

Products				
PlotID	Length	Product	Plies	Net Qty
B10	4-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
B11	14-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
B12	10-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
B13	10-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
B16	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	2
B17	9-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
J1	8-00-00	9 1/2" NI-20	1	15
J2	15-00-00	11 7/8" NI-20	1	37
J3	14-00-00	11 7/8" NI-20	2	4
J4	10-00-00	11 7/8" NI-20	1	14
J5	6-00-00	11 7/8" NI-20	1	2
J6	5-00-00	11 7/8" NI-20	1	7
J7	4-00-00	11 7/8" NI-20	1	3
J8	19-00-00	11 7/8" NI-40x	1	15
J9	19-00-00	11 7/8" NI-40x	2	4
xBk1	65-00-00	11 7/8" NI-20	1	1
xCa1	12-00-00	1 1/8" x 9 1/2" Rim Board	1	2
xCa2	12-00-00	1 1/8" x 11 7/8" Rim Board	1	17

Blocking panels are required over all interior supports under
load bearing walls or when floor joists are not continuous over
support.

Squash blocks are required under concentrated load.

Ceramic tile application as per O.B.C. 9.30.6

SUBFLOOR - 3/4" NAILED & GLUED *

RIMBOARD
1 - 1-1/8" X 11 7/8" O.S.B.

Connector Summary			
PlotID	Qty	Manuf	Product
H1	2		HUS1.81/10
H2	25		LT251188
H3	30		LT259

PL:109955



Yard:
Home Lumber

Contact:
Derek Frankfort

Date: June 20, 2020

ALPA ROOF
TRUSSES INC.

Gold Park

4205 (Broadview)

Pine Valley

LOCATION:
Vaughan

DESIGNER : FC

REVISIONS:

1	20210121
2	#2
3	#3
4	#4

LEGEND:

BBO	Beam By Others
APP	As Per Plan
PL(G)	Point Load from Girder
PL(W)	Point Load from Lintel
S.B.	Solid Bearing

DESIGN INFO:

JT/PL:

45147/106927

Layout ID:

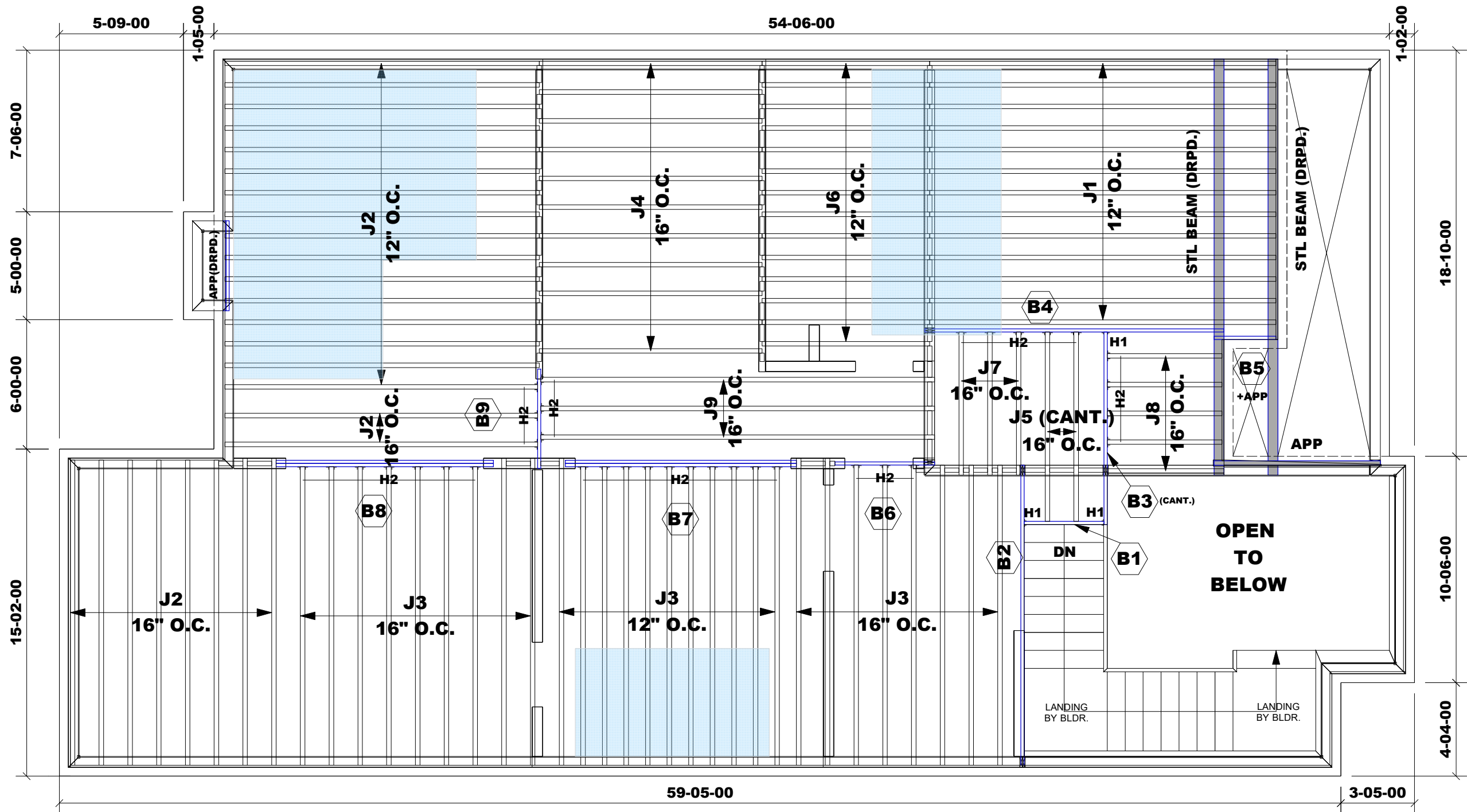
(321527)328366

LOADING

Live:	40.0 lb/ft²
Dead:	20.0 lb/ft²

DEFLECTION CRITERIA

Live:	360.000
Total	240.000



SECOND FLOOR FRAMING

UNIT : 4205 (BROADVIEW)
ELEV. " A "

Do not scale - refer to architectural
plans for dimensions

HATCH LEGEND	
	CERAMIC TILES

Products				
PlotID	Length	Product	Plies	Net Qty
B1	4-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
B2	14-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
B3	9-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
B4	14-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
B5	3-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
B6	5-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
B7	11-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2
B8	11-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2
B9	5-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
J1	17-00-00	11 7/8" NI-20	1	13
J2	15-00-00	11 7/8" NI-20	1	26
J3	14-00-00	11 7/8" NI-20	1	28
J4	11-00-00	11 7/8" NI-20	1	11
J5	9-00-00	11 7/8" NI-20	1	2
J6	8-00-00	11 7/8" NI-20	1	14
J7	7-00-00	11 7/8" NI-20	1	3
J8	6-00-00	11 7/8" NI-20	1	5
J9	19-00-00	11 7/8" NI-40x	1	3
xBk1	45-00-00	11 7/8" NI-20	1	1
xCa1	12-00-00	1 1/8" x 11 7/8" Rim Board	1	17

Blocking panels are required over all interior supports under
load bearing walls or when floor joists are not continuous over
support.

Squash blocks are required under concentrated load.

Ceramic tile application as per O.B.C. 9.30.6

SUBFLOOR - 3/4" NAILED & GLUED *

RIMBOARD
1 - 1-1/8" X 11 7/8" O.S.B.

Connector Summary			
PlotID	Qty	Manuf	Product
H1	3		HUS1.81/10
H2	35		LT251188

PL:109955



Yard:
Home Lumber

Contact:
Derek Frankfort

Date: June 20, 2020

ALPA ROOF
TRUSSES INC.

Gold Park

4205 (Broadview)

Pine Valley

LOCATION:
Vaughan

DESIGNER : FC

REVISIONS:

1	20210121
2	#2
3	#3
4	#4

LEGEND:

BBO	Beam By Others
APP	As Per Plan
PL(G)	Point Load from Girder
PL(W)	Point Load from Lintel
S.B.	Solid Bearing

DESIGN INFO:

JT/PL:

45147/106927

Layout ID:

(321527)328366

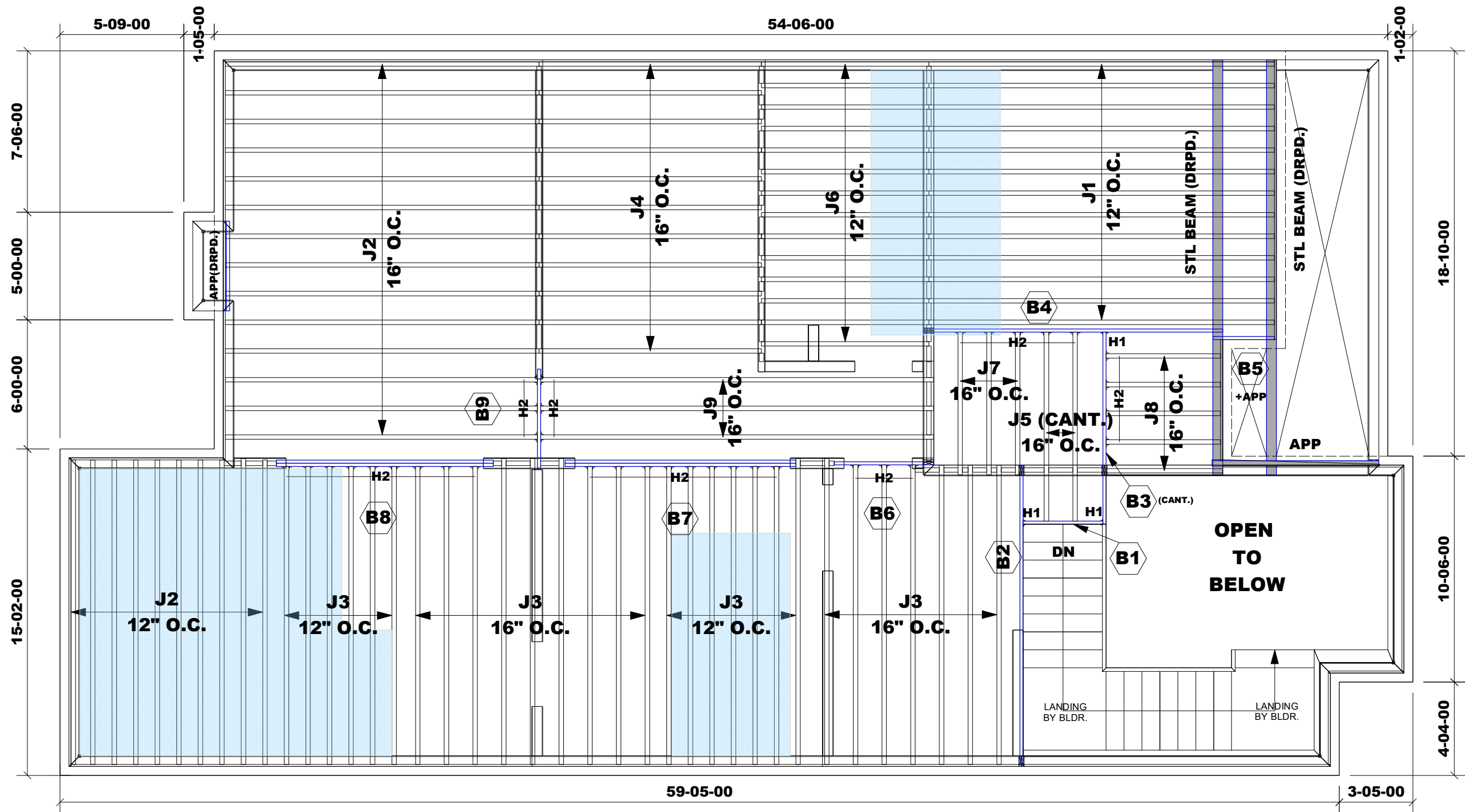
LOADING

Live:	40.0 lb/ft²
Dead:	20.0 lb/ft²

DEFLECTION CRITERIA

Live:	360.000
Total	240.000

5 OF 10



SECOND FLOOR FRAMING

UNIT : 4205 (BROADVIEW)

ELEV. " A "

ALT. 5 - BEDROOM

Do not scale - refer to architectural plans for dimensions

HATCH LEGEND	
	CERAMIC TILES

Products				
PlotID	Length	Product	Plies	Net Qty
B1	4-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
B2	14-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
B3	9-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
B4	14-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
B5	3-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
B6	5-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
B7	11-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2
B8	11-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2
B9	5-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
J1	17-00-00	11 7/8" NI-20	1	13
J2	15-00-00	11 7/8" NI-20	1	24
J3	14-00-00	11 7/8" NI-20	1	29
J4	11-00-00	11 7/8" NI-20	1	11
J5	9-00-00	11 7/8" NI-20	1	2
J6	8-00-00	11 7/8" NI-20	1	14
J7	7-00-00	11 7/8" NI-20	1	3
J8	6-00-00	11 7/8" NI-20	1	5
J9	19-00-00	11 7/8" NI-40x	1	3
xBk1	48-00-00	11 7/8" NI-20	1	1
xCa1	12-00-00	1 1/8" x 11 7/8" Rim Board	1	17

Blocking panels are required over all interior supports under load bearing walls or when floor joists are not continuous over support.

Squash blocks are required under concentrated load.

Ceramic tile application as per O.B.C. 9.30.6

SUBFLOOR - 3/4" NAILED & GLUED *

RIMBOARD
1 - 1-1/8" X 11 7/8" O.S.B.

Connector Summary			
PlotID	Qty	Manuf	Product
H1	3		HUS1.81/10
H2	36		LT251188

PL:109955



Yard:
Home Lumber

Contact:
Derek Frankfort

Date: June 20, 2020

ALPA ROOF
TRUSSES INC.

Gold Park

4205 (Broadview)

Pine Valley

LOCATION:
Vaughan

DESIGNER : FC

REVISIONS:

1	20210121
2	#2
3	#3
4	#4

LEGEND:

BBO	Beam By Others
APP	As Per Plan
PL(G)	Point Load from Girder
PL(W)	Point Load from Lintel
S.B.	Solid Bearing

DESIGN INFO:

JT/PL:

45147/106927

Layout ID:

(321527)328366

LOADING

Live:	40.0 lb/ft²
Dead:	20.0 lb/ft²

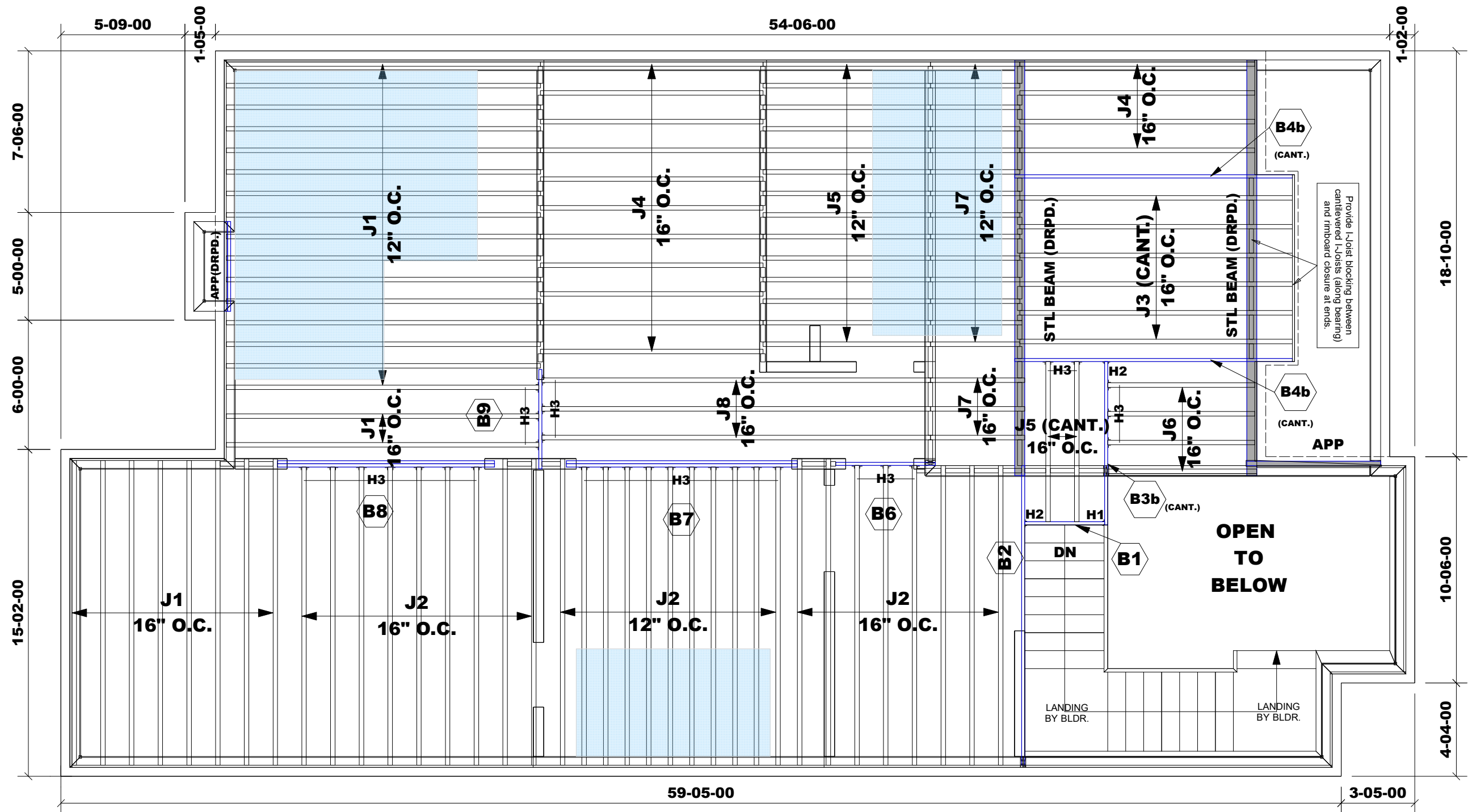
DEFLECTION CRITERIA

Live:	360.000
Total	240.000

6

OF

10



SECOND FLOOR FRAMING

UNIT : 4205 (BROADVIEW)

ELEV. " B "

Do not scale - refer to architectural plans for dimensions

HATCH LEGEND	
	CERAMIC TILES

Products					
PlotID	Length	Product	Plies	Net Qty	
B1	4-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1	
B2	14-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1	
B3b	8-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1	
B4b	13-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	2	
B6	5-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1	
B7	11-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2	
B8	11-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2	
B9	5-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1	
J1	15-00-00	11 7/8" NI-20	1	26	
J2	14-00-00	11 7/8" NI-20	1	28	
J3	13-00-00	11 7/8" NI-20	1	6	
J4	11-00-00	11 7/8" NI-20	1	15	
J5	8-00-00	11 7/8" NI-20	1	16	
J6	7-00-00	11 7/8" NI-20	1	4	
J7	5-00-00	11 7/8" NI-20	1	17	
J8	19-00-00	11 7/8" NI-40x	1	3	
xBk1	66-00-00	11 7/8" NI-20	1	1	
xCa1	12-00-00	1 1/8" x 11 7/8" Rim Board	1	17	

Blocking panels are required over all interior supports under load bearing walls or when floor joists are not continuous over support.

Squash blocks are required under concentrated load.

Ceramic tile application as per O.B.C. 9.30.6

SUBFLOOR - 3/4" NAILED & GLUED *

RIMBOARD
1 - 1-1/8" X 11 7/8" O.S.B.

Connector Summary			
PlotID	Qty	Manuf	Product
H1	1		HU9X(oOne flange concealed)
H2	2		HUS1.81/10
H3	31		LT251188

PL:109955



Yard:
Home Lumber

Contact:
Derek Frankfort

Date: June 20, 2020

ALPA ROOF
TRUSSES INC.

Gold Park

4205 (Broadview)

Pine Valley

LOCATION:
Vaughan

DESIGNER : FC

REVISIONS:

1	20210121
2	#2
3	#3
4	#4

LEGEND:

BBO	Beam By Others
APP	As Per Plan
PL(G)	Point Load from Girder
PL(W)	Point Load from Lintel
S.B.	Solid Bearing

DESIGN INFO:

JT/PL:

45147/106927

Layout ID:

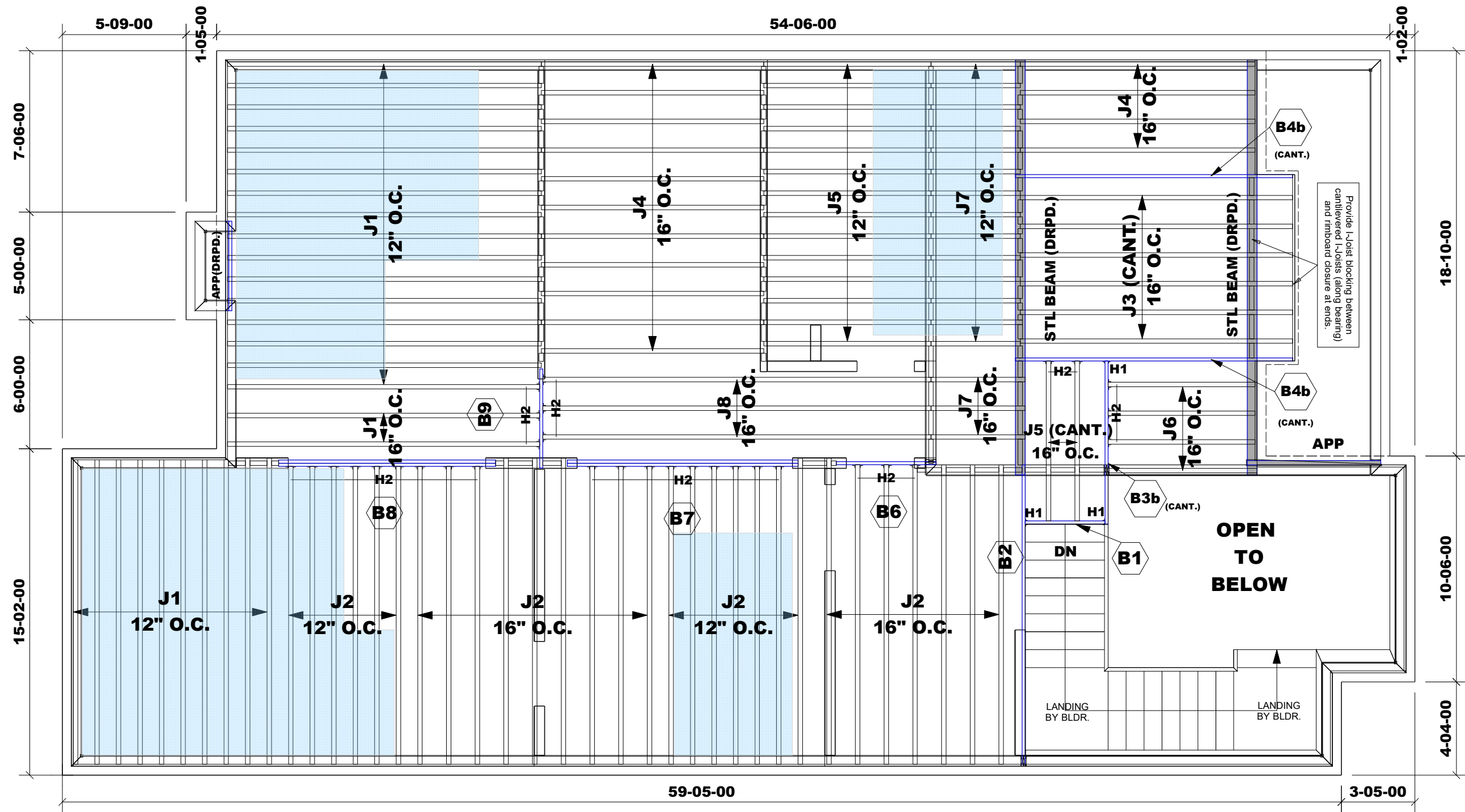
(321527)328366

LOADING

Live:	40.0 lb/ft²
Dead:	20.0 lb/ft²

DEFLECTION CRITERIA

Live:	360.000
Total	240.000



SECOND FLOOR FRAMING

UNIT : 4205 (BROADVIEW)

ELEV. " B "

ALT. 5 - BEDROOM

Do not scale - refer to architectural plans for dimensions

HATCH LEGEND	
	CERAMIC TILES

Products				
PlotID	Length	Product	Plies	Net Qty
B1	4-00-00	1-3/4" x 11-7/8" VERSA-LAM@ 2.0 3100 SP	1	1
B2	14-00-00	1-3/4" x 11-7/8" VERSA-LAM@ 2.0 3100 SP	1	1
B3b	8-00-00	1-3/4" x 11-7/8" VERSA-LAM@ 2.0 3100 SP	1	1
B4b	13-00-00	1-3/4" x 11-7/8" VERSA-LAM@ 2.0 3100 SP	1	2
B6	5-00-00	1-3/4" x 11-7/8" VERSA-LAM@ 2.0 3100 SP	1	1
B7	11-00-00	1-3/4" x 11-7/8" VERSA-LAM@ 2.0 3100 SP	2	2
B8	11-00-00	1-3/4" x 11-7/8" VERSA-LAM@ 2.0 3100 SP	2	2
B9	5-00-00	1-3/4" x 11-7/8" VERSA-LAM@ 2.0 3100 SP	1	1
J1	15-00-00	11 7/8" NI-20	1	28
J2	14-00-00	11 7/8" NI-20	1	29
J3	13-00-00	11 7/8" NI-20	1	6
J4	11-00-00	11 7/8" NI-20	1	15
J5	8-00-00	11 7/8" NI-20	1	16
J6	7-00-00	11 7/8" NI-20	1	4
J7	5-00-00	11 7/8" NI-20	1	17
J8	19-00-00	11 7/8" NI-40x	1	3
xBk1	66-00-00	11 7/8" NI-20	1	1
xCa1	12-00-00	1 1/8" x 11 7/8" Rim Board	1	17

Blocking panels are required over all interior supports under load bearing walls or when floor joists are not continuous over support.

Squash blocks are required under concentrated load.

Ceramic tile application as per O.B.C. 9.30.6

SUBFLOOR - 3/4" NAILED & GLUED *

RIMBOARD
1 - 1-1/8" X 11 7/8" O.S.B.

Connector Summary			
PlotID	Qty	Manuf	Product
H1	3		HUS1.81/10
H2	32		LT251188



Yard:
Home Lumber

Contact:
Derek Frankfort

Date: June 20, 2020

ALPA ROOF
TRUSSES INC.

Gold Park

4205 (Broadview)

Pine Valley

LOCATION:
Vaughan

DESIGNER : FC

REVISIONS:

1	20210121
2	#2
3	#3
4	#4

LEGEND:

BBO	Beam By Others
APP	As Per Plan
PL(G)	Point Load from Girder
PL(W)	Point Load from Lintel
S.B.	Solid Bearing

DESIGN INFO:

JT/PL:

45147/106927

Layout ID:

(321527)328366

LOADING

Live:	40.0 lb/ft²
Dead:	20.0 lb/ft²

DEFLECTION CRITERIA

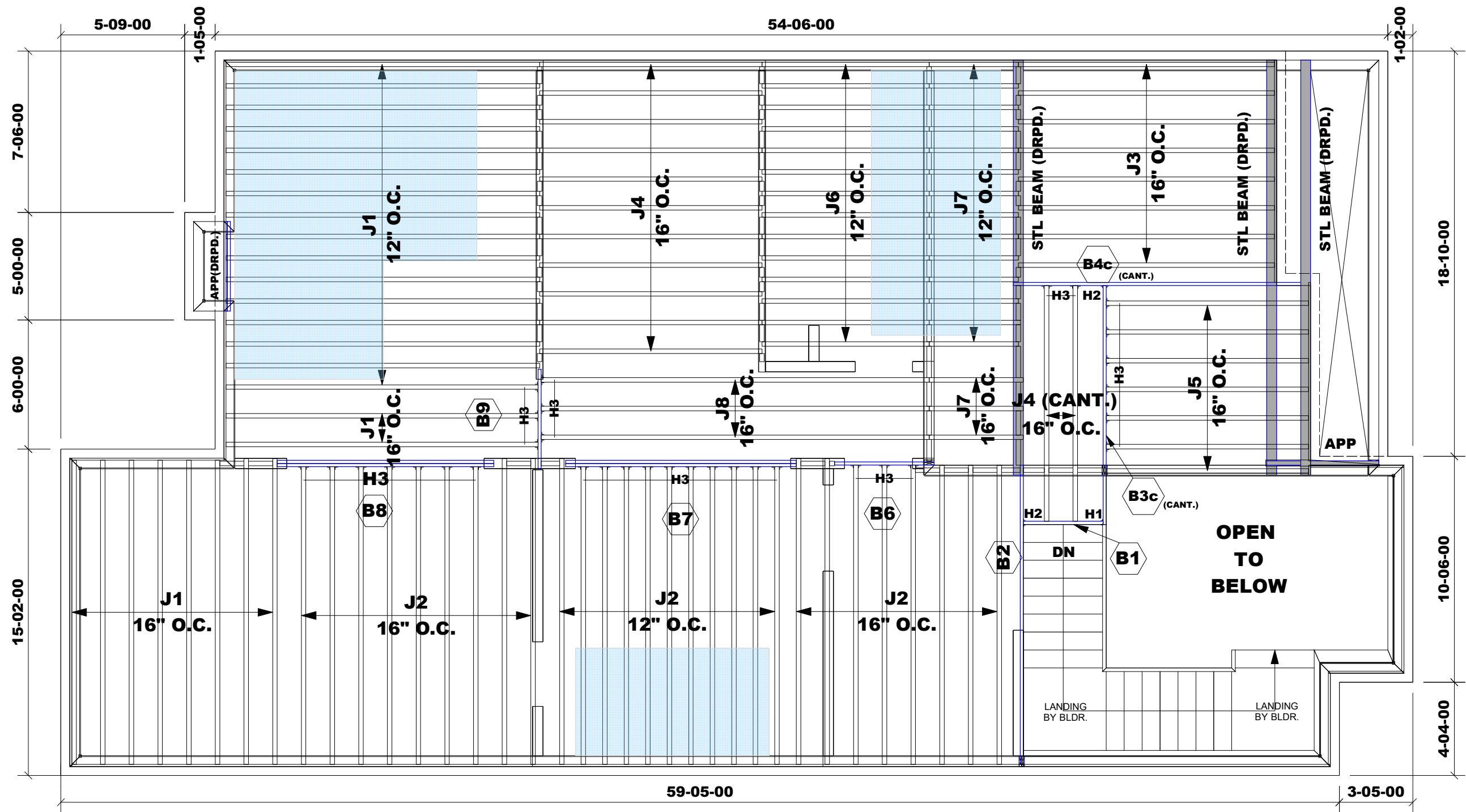
Live:	360.000
Total	240.000

8

OF

10

PL:109955



SECOND FLOOR FRAMING

UNIT : 4205 (BROADVIEW)

ELEV. " C "

Do not scale - refer to architectural plans for dimensions

HATCH LEGEND	
	CERAMIC TILES

Products				
PlotID	Length	Product	Plies	Net Qty
B1	4-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
B2	14-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
B3c	12-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
B4c	14-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
B6	5-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
B7	11-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2
B8	11-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2
B9	5-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
J1	15-00-00	11 7/8" NI-20	1	26
J2	14-00-00	11 7/8" NI-20	1	28
J3	12-00-00	11 7/8" NI-20	1	8
J4	11-00-00	11 7/8" NI-20	1	13
J5	10-00-00	11 7/8" NI-20	1	7
J6	8-00-00	11 7/8" NI-20	1	14
J7	5-00-00	11 7/8" NI-20	1	17
J8	19-00-00	11 7/8" NI-40x	1	3
xBk1	59-00-00	11 7/8" NI-20	1	1
xCa1	12-00-00	1 1/8" x 11 7/8" Rim Board	1	17

Blocking panels are required over all interior supports under load bearing walls or when floor joists are not continuous over support.

Squash blocks are required under concentrated load.

Ceramic tile application as per O.B.C. 9.30.6

SUBFLOOR - 3/4" NAILED & GLUED *

RIMBOARD
1 - 1-1/8" X 11 7/8" O.S.B.

Connector Summary			
PlotID	Qty	Manuf	Product
H1	1		HU9X(oOne flange concealed)
H2	2		HUS1.81/10
H3	34		LT251188

PL:109955



Yard:
Home Lumber

Contact:
Derek Frankfort

Date: June 20, 2020

ALPA ROOF
TRUSSES INC.

Gold Park

4205 (Broadview)

Pine Valley

LOCATION:
Vaughan

DESIGNER : FC

REVISIONS:

1	20210121
2	#2
3	#3
4	#4

LEGEND:

BBO	Beam By Others
APP	As Per Plan
PL(G)	Point Load from Girder
PL(W)	Point Load from Lintel
S.B.	Solid Bearing

DESIGN INFO:

JT/PL:

45147/106927

Layout ID:

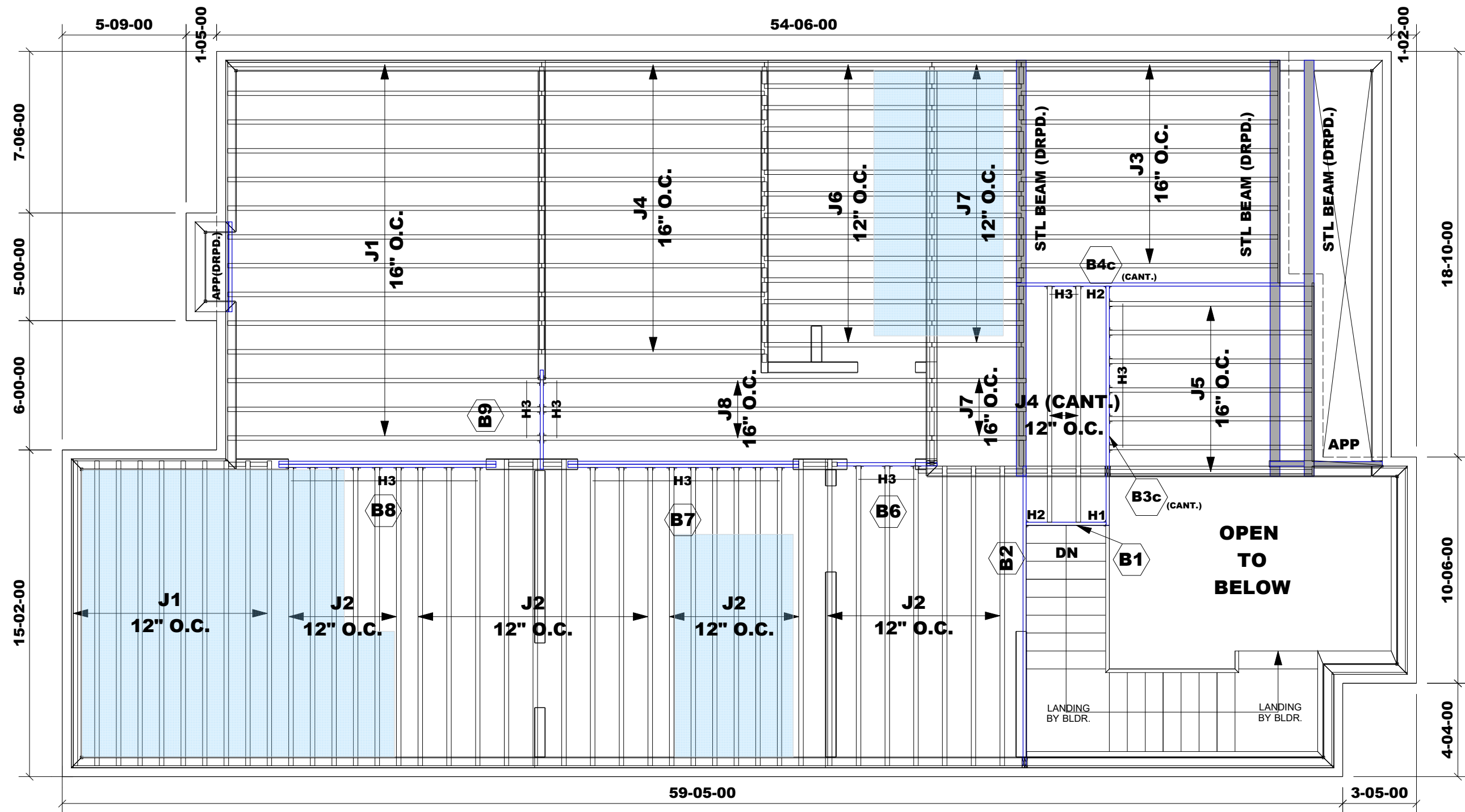
(321527)328366

LOADING

Live:	40.0 lb/ft²
Dead:	20.0 lb/ft²

DEFLECTION CRITERIA

Live:	360.000
Total	240.000



SECOND FLOOR FRAMING

UNIT : 4205 (BROADVIEW)

ELEV. " C "

ALT. 5 - BEDROOM

Do not scale - refer to architectural plans for dimensions

HATCH LEGEND	
	CERAMIC TILES

Products				
PlotID	Length	Product	Plies	Net Qty
B1	4-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
B2	14-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
B3c	12-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
B4c	14-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
B6	5-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
B7	11-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2
B8	11-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2
B9	5-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	1	1
J1	15-00-00	11 7/8" NI-20	1	24
J2	14-00-00	11 7/8" NI-20	1	29
J3	12-00-00	11 7/8" NI-20	1	8
J4	11-00-00	11 7/8" NI-20	1	13
J5	10-00-00	11 7/8" NI-20	1	7
J6	8-00-00	11 7/8" NI-20	1	14
J7	5-00-00	11 7/8" NI-20	1	17
J8	19-00-00	11 7/8" NI-40x	1	3
xBk1	61-00-00	11 7/8" NI-20	1	1
xCa1	12-00-00	1 1/8" x 11 7/8" Rim Board	1	17

Blocking panels are required over all interior supports under load bearing walls or when floor joists are not continuous over support.

Squash blocks are required under concentrated load.

Ceramic tile application as per O.B.C. 9.30.6

SUBFLOOR - 3/4" NAILED & GLUED *

RIMBOARD
1 - 1-1/8" X 11 7/8" O.S.B.

Connector Summary			
PlotID	Qty	Manuf	Product
H1	1		HU9X(oOne flange concealed)
H2	2		HUS1.81/10
H3	35		LT251188

PL:109955



Yard:
Home Lumber

Contact:
Derek Frankfort

Date: June 20, 2020

ALPA ROOF
TRUSSES INC.

Gold Park

4205 (Broadview)

Pine Valley

LOCATION:
Vaughan

DESIGNER : FC

REVISIONS:

1	20210121
2	#2
3	#3
4	#4

LEGEND:

BBO	Beam By Others
APP	As Per Plan
PL(G)	Point Load from Girder
PL(W)	Point Load from Lintel
S.B.	Solid Bearing

DESIGN INFO:

JT/PL:

45147/106927

Layout ID:

(321527)328366

LOADING

Live:	40.0 lb/ft²
Dead:	20.0 lb/ft²

DEFLECTION CRITERIA

Live:	360.000
Total	240.000

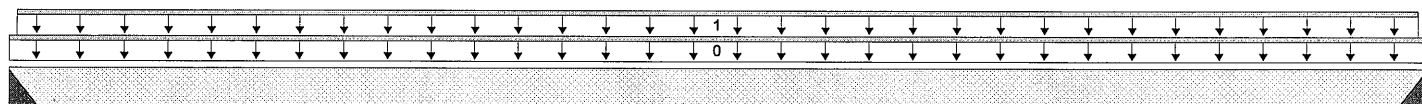
BC CALC® Member Report
 Build 7493
 Job name:
 Address:
 City, Province, Postal Code:
 Customer:
 Code reports:

Dry | 1 span | No cant.

June 20, 2020 12:20:46

File name: 4205 EL. A.mmdl
 Description: 2nd Floor - Supply/BOM\Flush Beams\B1(i26112)
 Specifier:
 Designer:
 Company:

CCMC 12472-R



B1

03-08-08

B2

Total Horizontal Product Length = 03-08-08

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B1, 2"	513 / 0	213 / 0		
B2, 2"	513 / 0	213 / 0		

Load Summary

Tag	Description	Load Type	Ref.	Start	End	Loc.	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Tributary
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	03-08-08	Top		6			00-00-00
1	User Load	Unf. Lin. (lb/ft)	L	00-00-04	03-08-04	Top	280	110			n/a

Controls Summary

	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	865 ft-lbs	17696 ft-lbs	4.9%	1	01-10-04
End Shear	394 lbs	7232 lbs	5.5%	1	01-01-14
Total Load Deflection	L/999 (0.003")	n/a	n/a	4	01-10-04
Live Load Deflection	L/999 (0.002")	n/a	n/a	5	01-10-04
Max Defl.	0.003"	n/a	n/a	4	01-10-04
Span / Depth	3.5				

Bearing Supports

	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1 Hanger	2" x 1-3/4"	1036 lbs	n/a	24.3%	HUS1.81/10
B2 Hanger	2" x 1-3/4"	1036 lbs	n/a	24.3%	HUS1.81/10

Cautions

Header for the hanger HUS1.81/10 is a Single 1-3/4" x 11-7/8" LVL Beam.
 Hanger model HUS1.81/10 and seat length were input by the user.

Notes

Design meets Code minimum (L/240) Total load deflection criteria.
 Design meets Code minimum (L/360) Live load deflection criteria.
 Calculations assume member is fully braced.
 Hanger Manufacturer: Unassigned
 Resistance Factor phi has been applied to all presented results per CSA O86.
 BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.
 Design based on Dry Service Condition.
 Importance Factor : Normal Part code : Part 9

Disclosure

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

BC CALC®, BC FRAMER®, AJS™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,

Single 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP
2nd Floor - Supply/BOM\Flush Beams\B2(i26031) (Flush Beam)

PASSED

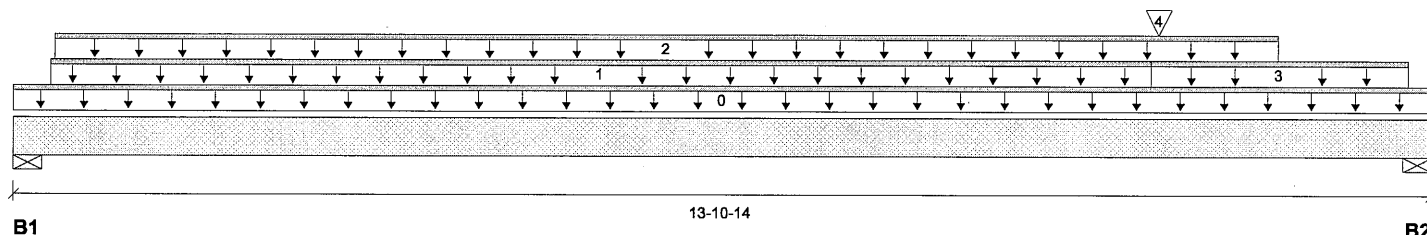
BC CALC® Member Report
 Build 7493
 Job name:
 Address:
 City, Province, Postal Code:
 Customer:
 Code reports:

CCMC 12472-R

Dry | 1 span | No cant.

June 20, 2020 12:20:46

File name: 4205 EL. A.mmdl
 Description: 2nd Floor - Supply/BOM\Flush Beams\B2(i26031)
 Specifier:
 Designer:
 Company:



Total Horizontal Product Length = 13-10-14

Reaction Summary (Down / Uplift) (lbs)

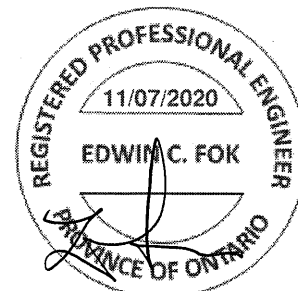
Bearing	Live	Dead	Snow	Wind
B1, 4-3/8"	240 / 0	807 / 0		
B2, 5-1/2"	625 / 0	880 / 0		

Load Summary

Tag	Description	Load Type	Ref.	Start	End	Loc.	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Tributary
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	13-10-14	Top		6			00-00-00
1	FC1 Floor Material	Unf. Lin. (lb/ft)	L	00-04-06	11-01-14	Top	22	11			n/a
2	13(i21016)	Unf. Lin. (lb/ft)	L	00-04-14	12-04-14	Top		101			n/a
3	FC1 Floor Material	Unf. Lin. (lb/ft)	L	11-01-14	13-08-02	Top	44	22			n/a
4	B1(i26112)	Conc. Pt. (lbs)	L	11-02-12	11-02-12	Top	513	213			n/a

Controls Summary

	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	3938 ft-lbs	11502 ft-lbs	34.2%	0	07-02-01
End Shear	1913 lbs	7232 lbs	26.5%	1	12-05-08
Total Load Deflection	L/610 (0.26")	n/a	39.3%	4	07-02-01
Live Load Deflection	L/999 (0.077")	n/a	n/a	5	07-05-12
Max Defl.	0.26"	n/a	n/a	4	07-02-01
Span / Depth	13.3				



Bearing Supports

	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Wall/Plate 4-3/8" x 1-3/4"	1130 lbs	36.9%	18.6%	Spruce-Pine-Fir
B2	Wall/Plate 5-1/2" x 1-3/4"	2038 lbs	34.4%	17.4%	Spruce-Pine-Fir

Notes

Design meets Code minimum (L/240) Total load deflection criteria.
 Design meets Code minimum (L/360) Live load deflection criteria.
 Calculations assume member is fully braced.
 Resistance Factor phi has been applied to all presented results per CSA O86.
 BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.
 Design based on Dry Service Condition.
 Importance Factor : Normal Part code : Part 9

Disclosure

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

BC CALC®, BC FRAMER®, AJS™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,

SG 020575

BC CALC® Member Report

Dry | 2 spans | L cant.

June 20, 2020 12:20:46

Build 7493

Job name:

File name: 4205 EL. A.mmdl

Address:

Description: 2nd Floor - Supply/BOM\Flush Beams\B3(i26045)

City, Province, Postal Code:

Specifier:

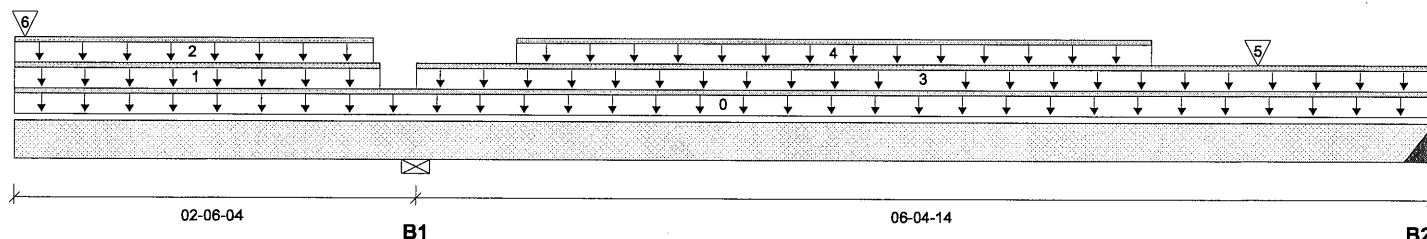
Customer:

Designer:

Code reports:

CCMC 12472-R

Company:



Total Horizontal Product Length = 08-11-02

Reaction Summary (Down / Uplift) (lbs)

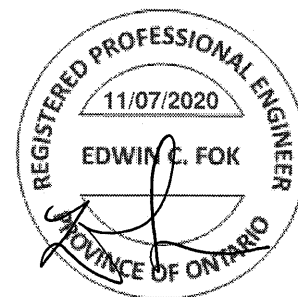
Bearing	Live	Dead	Snow	Wind
B1, 5-1/2"	1140 / 0	874 / 0		
B2, 2"	370 / 214	29 / 0		

Load Summary

Tag	Description	Load Type	Ref.	Start	End	Loc.	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Tributary
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	08-11-02	Top		6			00-00-00
1	FC1 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	02-03-08	Top	29	14			n/a
2	W5(i21018)	Unf. Lin. (lb/ft)	L	00-00-00	02-03-00	Top		26			n/a
3	FC1 Floor Material	Unf. Lin. (lb/ft)	L	02-06-04	08-11-02	Top	27	14			n/a
4	Smoothed Load	Unf. Lin. (lb/ft)	L	03-01-14	07-01-14	Top	104	52			n/a
5	J8(i26504)	Conc. Pt. (lbs)	L	07-09-14	07-09-14	Top	129	65			n/a
6	-	Conc. Pt. (lbs)	L	00-00-12	00-00-12	Top	513	390			n/a

Controls Summary

	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	847 ft-lbs	17696 ft-lbs	4.8%	6	06-05-14
Neg. Moment	-3409 ft-lbs	-17696 ft-lbs	19.3%	1	02-06-04
End Shear	501 lbs	7232 lbs	6.9%	3	07-09-04
Cont. Shear	1420 lbs	7232 lbs	19.6%	1	01-03-10
Total Load Deflection	2xL/1998 (0.057")	n/a	n/a	9	00-00-00
Live Load Deflection	2xL/1998 (0.035")	n/a	n/a	12	00-00-00
Total Neg. Defl.	L/999 (-0.017")	n/a	n/a	9	05-00-14
Max Defl.	-0.017"	n/a	n/a	9	05-00-14
Span / Depth	6.4				



Bearing Supports

	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Wall/Plate 5-1/2" x 1-3/4"	2802 lbs	47.3%	23.9%	Spruce-Pine-Fir
B2	Hanger 2" x 1-3/4"	592 lbs	n/a	13.9%	HUS1.81/10
B2	Uplift	295 lbs			

Cautions

Uplift of 294 lbs found at bearing B2.
Hanger B2 cannot handle uplift of -294 lbs.
Header for the hanger HUS1.81/10 is a Single 1-3/4" x 11-7/8" LVL Beam.
Hanger model HUS1.81/10 and seat length were input by the user. I

Long Cantilever: Sheathing required on bottom flange and adjacent back span or bracing designed by the design professional of record. Design professional of record must address uplift at supports.

BC CALC® Member Report
 Build 7493

Dry | 1 span | No cant.

June 20, 2020 12:20:46

Job name:

File name: 4205 EL. A.mmdl

Address:

Description: 2nd Floor - Supply/BOM\Flush Beams\B4(i26077)

City, Province, Postal Code:

Specifier:

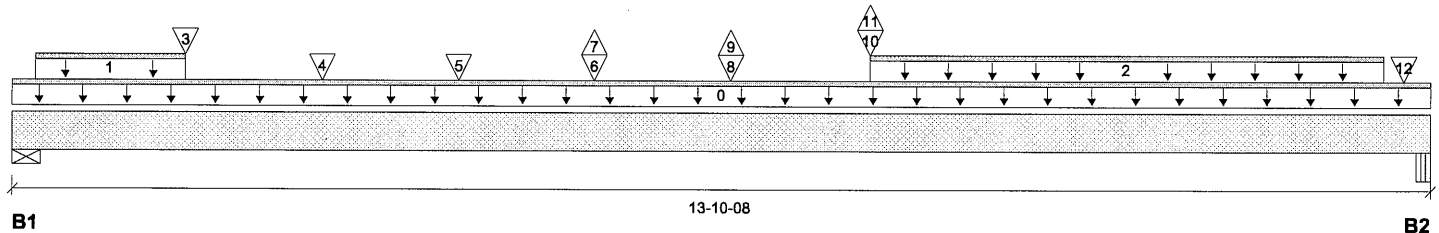
Customer:

Designer:

Code reports:

CCMC 12472-R

Company:



Total Horizontal Product Length = 13-10-08

Reaction Summary (Down / Uplift) (lbs)

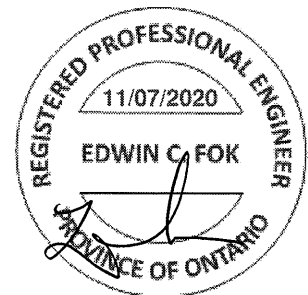
Bearing	Live	Dead	Snow	Wind
B1, 5-1/2"	799 / 111	366 / 0		
B2, 5-1/2"	585 / 155	266 / 0		

Load Summary

Tag	Description	Load Type	Ref.	Start	End	Loc.	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Tributary
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	13-10-08	Top		6			00-00-00
1	FC1 Floor Material	Unf. Lin. (lb/ft)	L	00-02-12	01-08-04	Top	25	12			n/a
2	FC1 Floor Material	Unf. Lin. (lb/ft)	L	08-04-10	13-05-00	Top	24	12			n/a
3	J7(i26386)	Conc. Pt. (lbs)	L	01-08-04	01-08-04	Top	161	81			n/a
4	J7(i26615)	Conc. Pt. (lbs)	L	03-00-04	03-00-04	Top	175	87			n/a
5	J7(i26200)	Conc. Pt. (lbs)	L	04-04-04	04-04-04	Top	175	87			n/a
6	J5(i26038)	Conc. Pt. (lbs)	L	05-08-04	05-08-04	Top	175	75			n/a
7	J5(i26038)	Conc. Pt. (lbs)	L	05-08-04	05-08-04	Top	-25				n/a
8	J5(i26061)	Conc. Pt. (lbs)	L	07-00-04	07-00-04	Top	177	75			n/a
9	J5(i26061)	Conc. Pt. (lbs)	L	07-00-04	07-00-04	Top	-26				n/a
10	B3(i26045)	Conc. Pt. (lbs)	L	08-04-10	08-04-10	Top	366	28			n/a
11	B3(i26045)	Conc. Pt. (lbs)	L	08-04-10	08-04-10	Top	-215				n/a
12	-	Conc. Pt. (lbs)	L	13-07-06	13-07-06	Top		39			n/a

Controls Summary

	Factored Demand	Factored Resistance	Demand/Resistance	Case	Location
Pos. Moment	5549 ft-lbs	17696 ft-lbs	31.4%	1	07-00-04
Neg. Moment	-354 ft-lbs	-17696 ft-lbs	2.0%	4	08-04-10
End Shear	1582 lbs	7232 lbs	21.9%	1	01-05-06
Total Load Deflection	L/662 (0.237")	n/a	36.2%	6	06-10-04
Live Load Deflection	L/916 (0.171")	n/a	39.3%	8	06-10-04
Max Defl.	0.237"	n/a	n/a	6	06-10-04
Span / Depth	13.2				



Bearing Supports

	Dim. (LxW)	Demand	Demand/Resistance Support	Demand/Resistance Member	Material
B1	Wall/Plate 5-1/2" x 1-3/4"	1656 lbs	28.0%	14.1%	Spruce-Pine-Fir
B2	Beam 5-1/2" x 1-3/4"	1210 lbs	20.4%	10.3%	Unspecified

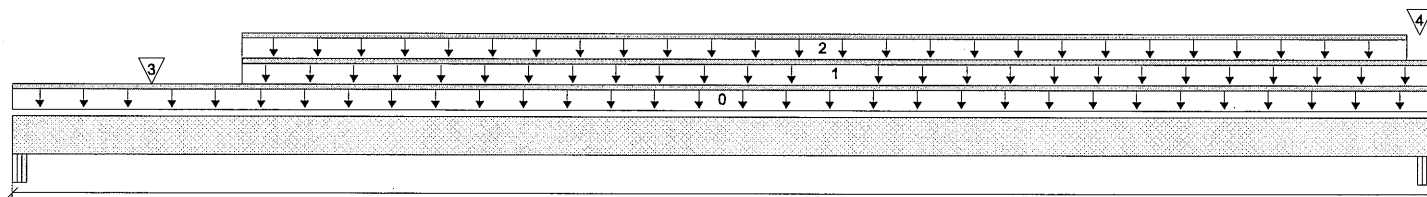
BC CALC® Member Report
 Build 7493
 Job name:
 Address:
 City, Province, Postal Code:
 Customer:
 Code reports:

Dry | 1 span | No cant.

June 20, 2020 12:20:46

File name: 4205 EL. A.mmdl
 Description: 2nd Floor - Supply/BOM\Flush Beams\B5(i26644)
 Specifier:
 Designer:
 Company:

CCMC 12472-R



B1

02-10-06

B2

Total Horizontal Product Length = 02-10-06

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B1, 5-1/2"	3 / 0	252 / 0	182 / 0	
B2, 4-3/8"	4 / 0	308 / 0	247 / 0	

Load Summary

Tag	Description	Load Type	Ref.	Start	End	Loc.	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Tributary
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	02-10-06	Top		6			00-00-00
1	FC1 Floor Material	Unf. Lin. (lb/ft)	L	00-05-08	02-10-06	Top	3				n/a
2	6(i21003)	Unf. Lin. (lb/ft)	L	00-05-08	02-09-12	Top		215	178		n/a
3	-	Conc. Pt. (lbs)	L	00-03-06	00-03-06	Top		22			n/a
4	6(i21003)	Conc. Pt. (lbs)	L	02-10-01	02-10-01	Top		11	9		n/a

Controls Summary

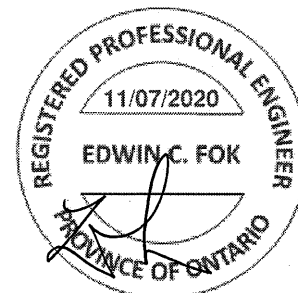
	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	322 ft-lbs	17696 ft-lbs	1.8%	13	01-05-12
End Shear	18 lbs	7232 lbs	0.2%	13	01-05-06
Total Load Deflection	L/999 (0")	n/a	n/a	35	01-05-12
Live Load Deflection	L/999 (0")	n/a	n/a	51	01-05-12
Max Defl.	0"	n/a	n/a	35	01-05-12
Span / Depth	2.2				

Bearing Supports

	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Beam 5-1/2" x 1-3/4"	592 lbs	10.0%	5.0%	Unspecified
B2	Beam 4-3/8" x 1-3/4"	760 lbs	16.1%	8.1%	Unspecified

Notes

Design meets Code minimum (L/240) Total load deflection criteria.
 Design meets Code minimum (L/360) Live load deflection criteria.
 Calculations assume member is fully braced.
 Resistance Factor phi has been applied to all presented results per CSA O86.
 BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.
 Unbalanced snow loads determined from building geometry were used in selected product's verification.
 Design based on Dry Service Condition.
 Importance Factor : Normal Part code : Part 9



Disclosure

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BC CALC®, BC FRAMER®, AJS™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,

Single 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

2nd Floor - Supply/BOM\Flush Beams\B6(i26153) (Flush Beam)

PASSED

BC CALC® Member Report
Build 7493

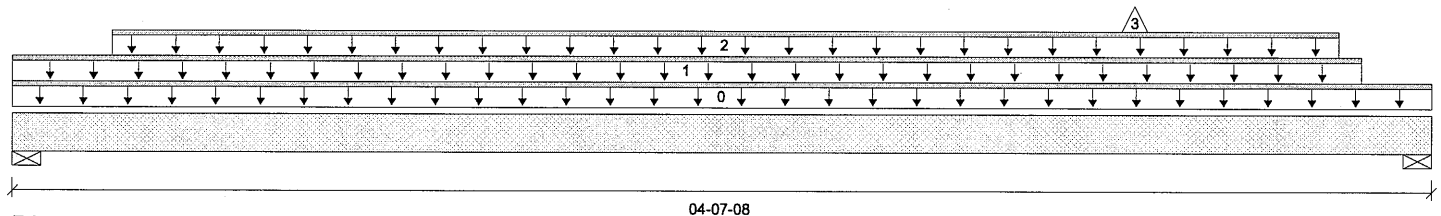
Dry | 1 span | No cant.

June 20, 2020 12:20:46

Job name:
Address:
City, Province, Postal Code:
Customer:
Code reports:

CCMC 12472-R

File name: 4205 EL. A.mmdl
Description: 2nd Floor - Supply/BOM\Flush Beams\B6(i26153)
Specifier:
Designer:
Company:



Total Horizontal Product Length = 04-07-08

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B1, 5-1/2"	603 / 0	316 / 0		
B2, 5-1/2"	600 / 2	313 / 0		

Load Summary

Tag	Description	Load Type	Ref.	Start	End	Loc.	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Tributary
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	04-07-08	Top		6			00-00-00
1	FC1 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	04-04-12	Top	24	12			n/a
2	Smoothed Load	Unf. Lin. (lb/ft)	L	00-03-14	04-03-14	Top	274	137			n/a
3	J3(i26394)	Conc. Pt. (lbs)	L	03-07-14	03-07-14	Top	-2				n/a

Controls Summary

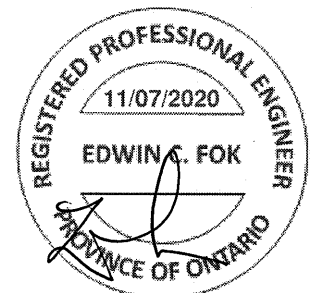
	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	1308 ft-lbs	17696 ft-lbs	7.4%	1	02-03-14
End Shear	852 lbs	7232 lbs	11.8%	1	01-05-06
Total Load Deflection	L/999 (0.005")	n/a	n/a	6	02-03-14
Live Load Deflection	L/999 (0.003")	n/a	n/a	8	02-03-14
Max Defl.	0.005"	n/a	n/a	6	02-03-14
Span / Depth	3.9				

Bearing Supports

	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Wall/Plate 5-1/2" x 1-3/4"	1300 lbs	22.0%	11.1%	Spruce-Pine-Fir
B2	Wall/Plate 5-1/2" x 1-3/4"	1292 lbs	21.8%	11.0%	Spruce-Pine-Fir

Notes

Design meets Code minimum (L/240) Total load deflection criteria.
Design meets Code minimum (L/360) Live load deflection criteria.
Calculations assume member is fully braced.
Resistance Factor phi has been applied to all presented results per CSA O86.
BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.
Design based on Dry Service Condition.
Importance Factor : Normal Part code : Part 9



Disclosure

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BC CALC®, BC FRAMER®, AJS™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,

SG020579

Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP 2nd Floor - Supply/BOM\Flush Beams\B7(i26442) (Flush Beam)

PASSED

BC CALC® Member Report

Dry | 1 span | No cant.

June 20, 2020 12:20:46

Build 7493

Job name:

File name: 4205 EL. A.mmdl

Address:

Description: 2nd Floor - Supply/BOM\Flush Beams\B7(i26442)

City, Province, Postal Code:

Specifier:

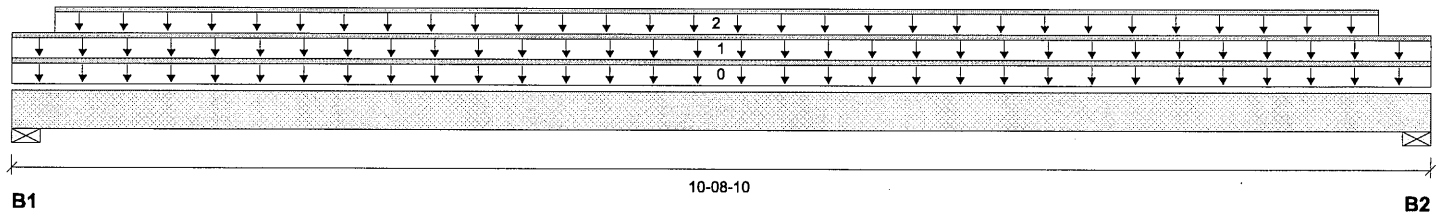
Customer:

Designer:

Code reports:

CCMC 12472-R

Company:



Total Horizontal Product Length = 10-08-10

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B1, 5-1/2"	1555 / 0	841 / 0		
B2, 3-1/8"	1476 / 0	799 / 0		

Load Summary

Tag	Description	Load Type	Ref.	Start	End	Loc.	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Tributary
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	10-08-10	Top		12			00-00-00
1	FC1 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	10-08-10	Top	24	12			n/a
2	Smoothed Load	Unf. Lin. (lb/ft)	L	00-03-14	10-03-14	Top	277	138			n/a

Controls Summary

	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	8414 ft-lbs	35392 ft-lbs	23.8%	1	05-09-14
End Shear	2919 lbs	14464 lbs	20.2%	1	09-05-10
Total Load Deflection	L/999 (0.113")	n/a	n/a	4	05-05-06
Live Load Deflection	L/999 (0.073")	n/a	n/a	5	05-05-06
Max Defl.	0.113"	n/a	n/a	4	05-05-06
Span / Depth	10.2				

Bearing Supports

	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Wall/Plate 5-1/2" x 3-1/2"	3383 lbs	28.6%	14.4%	Spruce-Pine-Fir
B2	Wall/Plate 3-1/8" x 3-1/2"	3213 lbs	47.8%	24.1%	Spruce-Pine-Fir

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

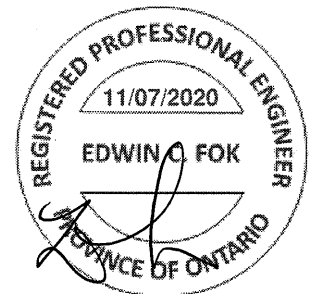
Calculations assume unbraced length of Top: 00-00-00, Bottom: 00-00-00.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor : Normal Part code : Part 9

Nail one ply to another with
3 1/2" spiral nails @ 10"
o.c., staggered in 2 rows


BC CALC® Member Report

Dry | 1 span | No cant.

June 20, 2020 12:20:46

Build 7493

Job name:

File name: 4205 EL. A.mmdl

Address:

Description: 2nd Floor - Supply/BOM\Flush Beams\B8(i26161)

City, Province, Postal Code:

Specifier:

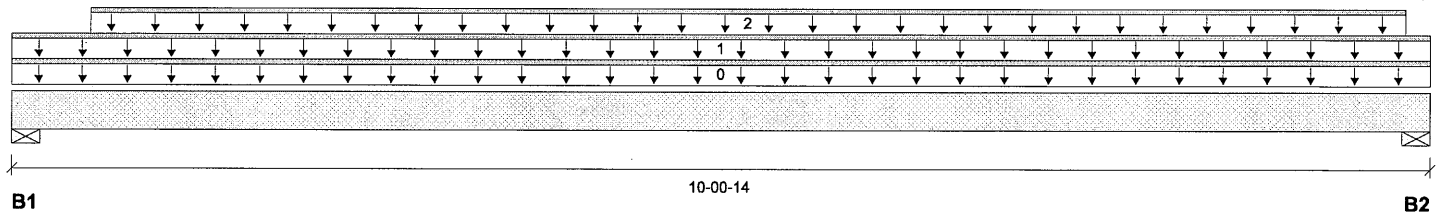
Customer:

Designer:

Code reports:

CCMC 12472-R

Company:



Total Horizontal Product Length = 10-00-14

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B1, 5-3/8"	1326 / 0	725 / 0		
B2, 5-1/2"	1436 / 0	780 / 0		

Load Summary

Tag	Description	Load Type	Ref.	Start	End	Loc.	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Tributary
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	10-00-14	Top		12			00-00-00
1	FC1 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	10-00-14	Top	18	9			n/a
2	Smoothed Load	Unf. Lin. (lb/ft)	L	00-06-12	09-10-12	Top	277	139			n/a

Controls Summary

	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	7027 ft-lbs	35392 ft-lbs	19.9%	1	05-02-12
End Shear	2654 lbs	14464 lbs	18.3%	1	01-05-04
Total Load Deflection	L/999 (0.078")	n/a	n/a	4	05-00-12
Live Load Deflection	L/999 (0.051")	n/a	n/a	5	05-00-12
Max Defl.	0.078"	n/a	n/a	4	05-00-12
Span / Depth	9.4				

Bearing Supports

	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Wall/Plate 5-3/8" x 3-1/2"	2895 lbs	25.0%	12.6%	Spruce-Pine-Fir
B2	Wall/Plate 5-1/2" x 3-1/2"	3129 lbs	26.4%	13.3%	Spruce-Pine-Fir

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

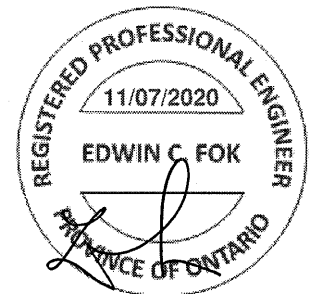
Design based on Dry Service Condition.

Importance Factor : Normal Part code : Part 9

Nail one ply to another with

3 1/2" spiral nails @ 10"

o.c, staggered in 2 rows

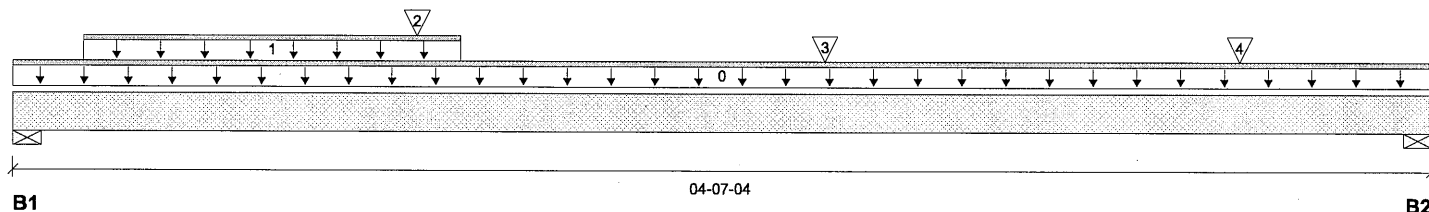


BC CALC® Member Report
 Build 7493
 Job name:
 Address:
 City, Province, Postal Code:
 Customer:
 Code reports: CCMC 12472-R

Dry | 1 span | No cant.

June 20, 2020 12:20:46

File name: 4205 EL. A.mmdl
 Description: 2nd Floor - Supply/BOM\Flush Beams\B9(i26493)
 Specifier:
 Designer:
 Company:



Total Horizontal Product Length = 04-07-04

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B1, 5-1/2"	1020 / 0	523 / 0		
B2, 5-1/2"	1489 / 0	757 / 0		

Load Summary

Tag	Description	Load Type	Ref.	Start	End	Loc.	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Tributary
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	04-07-04	Top		6			00-00-00
1	FC1 Floor Material	Unf. Lin. (lb/ft)	L	00-02-12	01-05-06	Top	41	21			n/a
2	-	Conc. Pt. (lbs)	L	01-03-11	01-03-11	Top	762	380			n/a
3	-	Conc. Pt. (lbs)	L	02-07-10	02-07-10	Top	868	433			n/a
4	-	Conc. Pt. (lbs)	L	03-11-12	03-11-12	Top	829	414			n/a

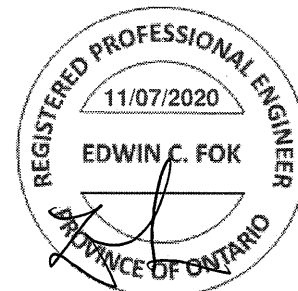
Controls Summary

	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	2442 ft-lbs	17696 ft-lbs	13.8%	1	02-05-06
End Shear	1923 lbs	7232 lbs	26.6%	1	01-05-06
Total Load Deflection	L/999 (0.009")	n/a	n/a	4	02-03-14
Live Load Deflection	L/999 (0.006")	n/a	n/a	5	02-03-14
Max Defl.	0.009"	n/a	n/a	4	02-03-14
Span / Depth	3.9				

Bearing Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Wall/Plate 5-1/2" x 1-3/4"	2183 lbs	36.9%	18.6%	Spruce-Pine-Fir
B2	Wall/Plate 5-1/2" x 1-3/4"	3180 lbs	53.7%	27.1%	Spruce-Pine-Fir

Notes

Design meets Code minimum (L/240) Total load deflection criteria.
 Design meets Code minimum (L/360) Live load deflection criteria.
 Calculations assume member is fully braced.
 Resistance Factor phi has been applied to all presented results per CSA O86.
 BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.
 Design based on Dry Service Condition.
 Importance Factor : Normal Part code : Part 9



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BC CALC®, BC FRAMER®, AJS™, ALLJOIST®, BC RIM BOARD™, BCi®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,

Build 7493

Job name:

File name: 4205 EL. A.mmdl

Address:

Description: Ground Floor - Supply/BOM\Flush Beams\B10(i27774)

City, Province, Postal Code:

Specifier:

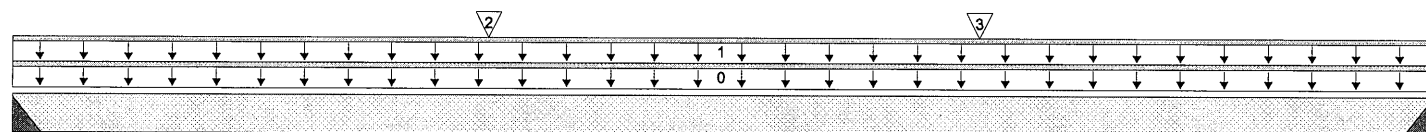
Customer:

Designer:

Code reports:

CCMC 12472-R

Company:



B1

03-10-04

B2

Total Horizontal Product Length = 03-10-04

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B1, 2"	643 / 0	278 / 0		
B2, 2"	648 / 0	287 / 0		

Load Summary

Tag	Description	Load Type	Ref.	Start	End	Loc.	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Tributary
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	03-10-04	Top		6			00-00-00
1	User Load	Unf. Lin. (lb/ft)	L	00-00-00	03-10-04	Top	240	90			n/a
2	J7(i28061)	Conc. Pt. (lbs)	L	01-03-08	01-03-08	Top	185	92			n/a
3	J7(i28062)	Conc. Pt. (lbs)	L	02-07-08	02-07-08	Top	181	99			n/a

Controls Summary

	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	1253 ft-lbs	17696 ft-lbs	7.1%	1	01-11-00
End Shear	771 lbs	7232 lbs	10.7%	1	02-08-06
Total Load Deflection	L/999 (0.004")	n/a	n/a	4	01-11-00
Live Load Deflection	L/999 (0.003")	n/a	n/a	5	01-11-00
Max Defl.	0.004"	n/a	n/a	4	01-11-00
Span / Depth	3.7				

Bearing Supports

	Dim. (LxW)	Demand	Demand/ Resistance	Demand/ Resistance	Member	Material
B1	Hanger 2" x 1-3/4"	1312 lbs	n/a	30.7%		HUS1.81/10
B2	Hanger 2" x 1-3/4"	1331 lbs	n/a	31.2%		HUS1.81/10

Cautions

Header for the hanger HUS1.81/10 is a Single 1-3/4" x 11-7/8" LVL Beam.

Hanger model HUS1.81/10 and seat length were input by the user.

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

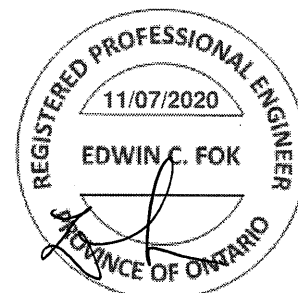
Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

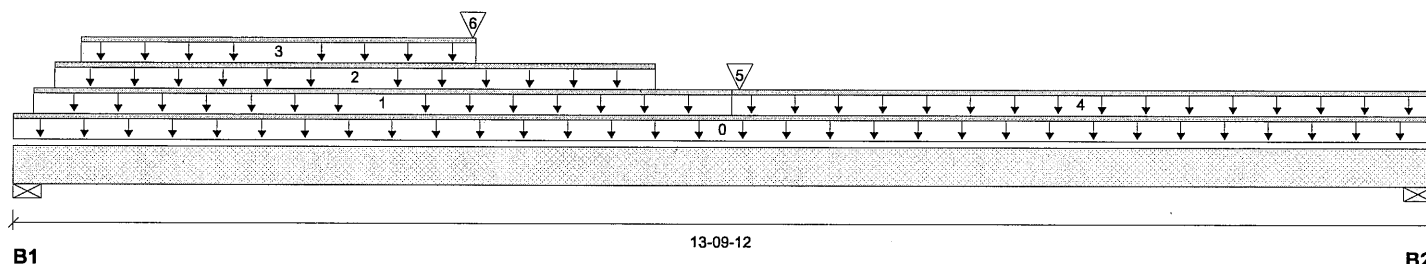
Design based on Dry Service Condition.

Importance Factor : Normal Part code : Part 9


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BC CALC®, BC FRAMER®, AJS™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,


Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B1, 2-3/8"	989 / 0	761 / 0		
B2, 2-3/8"	788 / 0	486 / 0		

Load Summary

Tag	Description	Load Type	Ref.	Start	End	Loc.	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Tributary
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	13-09-12	Top		6			00-00-00
1	FC2 Floor Material	Unf. Lin. (lb/ft)	L	00-02-06	06-11-14	Top	22	11			n/a
2	17(i21048)	Unf. Lin. (lb/ft)	L	00-04-14	06-02-14	Top		68			n/a
3	User Load	Unf. Lin. (lb/ft)	L	00-07-14	04-05-14	Top	70	27			n/a
4	FC2 Floor Material	Unf. Lin. (lb/ft)	L	06-11-14	13-09-12	Top	48	24			n/a
5	B10(i27774)	Conc. Pt. (lbs)	L	07-00-12	07-00-12	Top	643	278			n/a
6	User Load	Conc. Pt. (lbs)	L	04-05-08	04-05-08	Top	385	145			n/a

Controls Summary

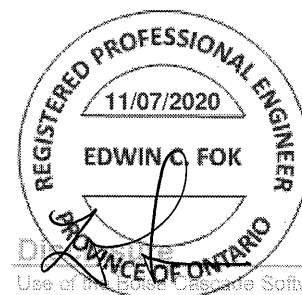
	Factored Demand	Factored Resistance	Demand/Resistance	Case	Location
Pos. Moment	9335 ft-lbs	17696 ft-lbs	52.8%	1	07-00-12
End Shear	2378 lbs	7232 lbs	32.9%	1	01-02-04
Total Load Deflection	L/389 (0.418")	n/a	61.8%	4	06-09-10
Live Load Deflection	L/638 (0.255")	n/a	56.5%	5	06-09-10
Max Defl.	0.418"	n/a	n/a	4	06-09-10
Span / Depth	13.7				

Bearing Supports

	Dim. (LxW)	Demand	Demand/Resistance Support	Demand/Resistance Member	Material
B1	Wall/Plate 2-3/8" x 1-3/4"	2435 lbs	95.2%	48.0%	Spruce-Pine-Fir
B2	Wall/Plate 2-3/8" x 1-3/4"	1789 lbs	70.0%	35.3%	Spruce-Pine-Fir

Notes

Design meets Code minimum (L/240) Total load deflection criteria.
 Design meets Code minimum (L/360) Live load deflection criteria.
 Calculations assume member is fully braced.
 Resistance Factor phi has been applied to all presented results per CSA O86.
 BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.
 Design based on Dry Service Condition.
 Importance Factor : Normal Part code : Part 9



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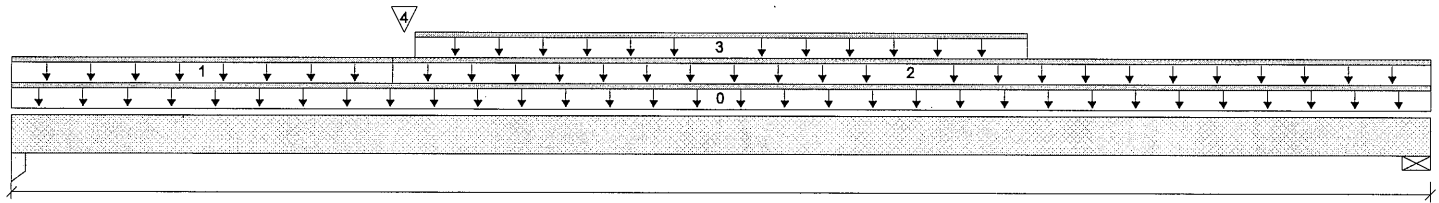
BC CALC® Member Report
 Build 7493

Dry | 1 span | No cant.

June 20, 2020 12:28:05

 Job name:
 Address:
 City, Province, Postal Code:
 Customer:
 Code reports:

CCMC 12472-R

 File name: 4205 EL. A.mmdl
 Description: Ground Floor - Supply/BOM\Flush Beams\B12(i28187)
 Specifier:
 Designer:
 Company:


B1

09-03-14

B2

Total Horizontal Product Length = 09-03-14

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B1, 3-1/2"	600 / 0	348 / 0		
B2, 2-3/8"	334 / 0	231 / 0		

Load Summary

Tag	Description	Load Type	Ref.	Start	End	Loc.	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Tributary
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	09-03-14	Top		6			00-00-00
1	FC2 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	02-06-00	Top	13	6			n/a
2	FC2 Floor Material	Unf. Lin. (lb/ft)	L	02-06-00	09-03-14	Top	37	19			n/a
3	FC2 Floor Material	Unf. Lin. (lb/ft)	L	02-07-12	06-08-00	Top		23			n/a
4	B10(i27774)	Conc. Pt. (lbs)	L	02-06-14	02-06-14	Top	648	287			n/a

Controls Summary

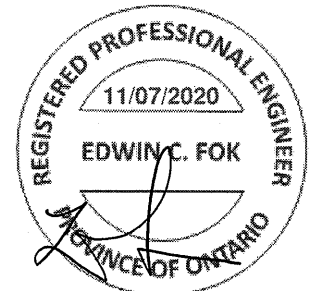
	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	3012 ft-lbs	17696 ft-lbs	17.0%	1	02-06-14
End Shear	1290 lbs	7232 lbs	17.8%	1	01-03-06
Total Load Deflection	L/999 (0.057")	n/a	n/a	4	04-04-14
Live Load Deflection	L/999 (0.035")	n/a	n/a	5	04-04-14
Max Defl.	0.057"	n/a	n/a	4	04-04-14
Span / Depth	9.1				

Bearing Supports

	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Column 3-1/2" x 1-3/4"	1334 lbs	20.9%	17.9%	Unspecified
B2	Wall/Plate 2-3/8" x 1-3/4"	791 lbs	30.9%	15.6%	Spruce-Pine-Fir

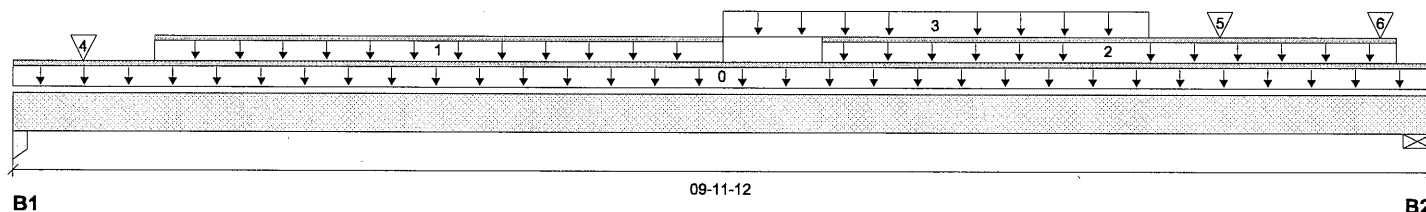
Notes

Design meets Code minimum (L/240) Total load deflection criteria.
 Design meets Code minimum (L/360) Live load deflection criteria.
 Calculations assume member is fully braced.
 Resistance Factor phi has been applied to all presented results per CSA O86.
 BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.
 Design based on Dry Service Condition.
 Importance Factor : Normal Part code : Part 9


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Total Horizontal Product Length = 09-11-12

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B1, 1-3/4"	1058 / 0	545 / 0		
B2, 5-1/2"	1783 / 0	1502 / 0		

Load Summary

Tag	Description	Load Type	Ref.	Start	End	Loc.	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Tributary
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	09-11-12	Top		6			00-00-00
1	Smoothed Load	Unf. Lin. (lb/ft)	L	00-11-14	04-11-14	Top	187	94			n/a
2	User Load	Unf. Lin. (lb/ft)	L	05-08-04	09-08-12	Top	240	90			n/a
3	Smoothed Load	Trapezoidal (lb/ft)	L	04-11-14	07-11-14	Top	187	108			n/a
4	J6(i28075)	Conc. Pt. (lbs)	L	00-05-14	00-05-14	Top	149	75			n/a
5	J6(i28067)	Conc. Pt. (lbs)	L	08-05-14	08-05-14	Top	187	94			n/a
6	-	Conc. Pt. (lbs)	L	09-07-06	09-07-06	Top	226	775			n/a

Controls Summary

	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	6578 ft-lbs	17696 ft-lbs	37.2%	1	05-05-14
End Shear	2667 lbs	7232 lbs	36.9%	1	08-06-06
Total Load Deflection	L/748 (0.152")	n/a	32.1%	4	04-11-14
Live Load Deflection	L/999 (0.102")	n/a	n/a	5	04-11-14
Max Defl.	0.152"	n/a	n/a	4	04-11-14
Span / Depth	9.6				

Bearing Supports

	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Column 1-3/4" x 1-3/4"	2267 lbs	70.9%	60.7%	Unspecified
B2	Wall/Plate 5-1/2" x 1-3/4"	4552 lbs	76.9%	38.8%	Spruce-Pine-Fir

Notes

Design meets Code minimum (L/240) Total load deflection criteria.
Design meets Code minimum (L/360) Live load deflection criteria.
Calculations assume member is fully braced.
Resistance Factor phi has been applied to all presented results per CSA O86.
BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.
Design based on Dry Service Condition.
Importance Factor : Normal Part code : Part 9



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Build 7493

Job name:

File name: 4205 EL. A.mmdl

Address:

Description: Ground Floor - Supply/BOM\Flush Beams\B14(i27565)

City, Province, Postal Code:

Specifier:

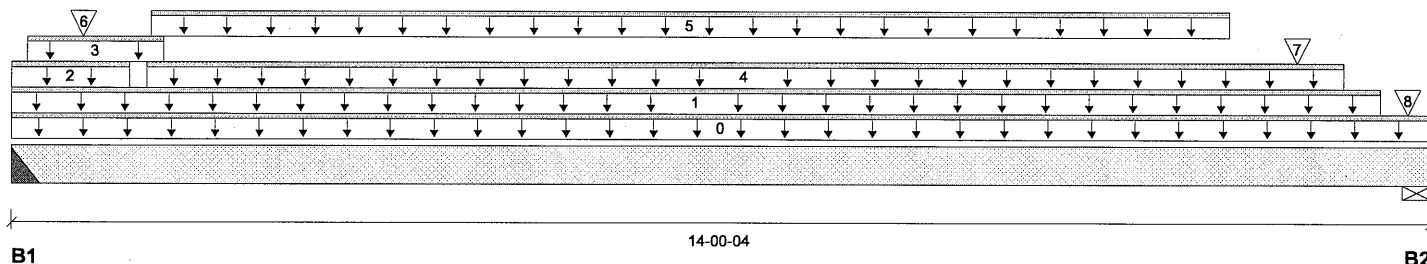
Customer:

Designer:

Code reports:

CCMC 12472-R

Company:



Total Horizontal Product Length = 14-00-04

Reaction Summary (Down / Uplift) (lbs)

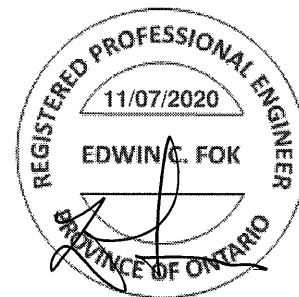
Bearing	Live	Dead	Snow	Wind
B1, 2"	3946 / 0	2572 / 0		
B2, 3-1/2"	3683 / 0	2514 / 0		

Load Summary

Tag	Description	Load Type	Ref.	Start	End	Loc.	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Tributary
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	14-00-04	Top		18			00-00-00
1	19(i21055)	Unf. Lin. (lb/ft)	L	00-00-00	13-06-04	Top		68			n/a
2	19(i21055)	Unf. Lin. (lb/ft)	L	00-00-00	01-01-14	Top	207	104			n/a
3	19(i21055)	Unf. Lin. (lb/ft)	L	00-01-14	01-05-14	Top	154	76			n/a
4	19(i21055)	Unf. Lin. (lb/ft)	L	01-03-14	13-01-14	Top	366	183			n/a
5	Smoothed Load	Unf. Lin. (lb/ft)	L	01-04-06	12-00-06	Top	214	108			n/a
6	J5(i27967)	Conc. Pt. (lbs)	L	00-08-06	00-08-06	Top	248	124			n/a
7	J5(i27968)	Conc. Pt. (lbs)	L	12-08-06	12-08-06	Top	268	132			n/a
8	E8(i21023)	Conc. Pt. (lbs)	L	13-09-08	13-09-08	Top		112			n/a

Controls Summary

	Factored Demand	Factored Resistance	Demand/Resistance	Case	Location
Pos. Moment	31302 ft-lbs	55212 ft-lbs	56.7%	1	07-00-06
End Shear	8418 lbs	21696 lbs	38.8%	1	12-08-14
Total Load Deflection	L/320 (0.514")	n/a	75.1%	4	06-10-06
Live Load Deflection	L/527 (0.312")	n/a	68.3%	5	06-10-06
Max Defl.	0.514"	n/a	n/a	4	06-10-06
Span / Depth	13.8				

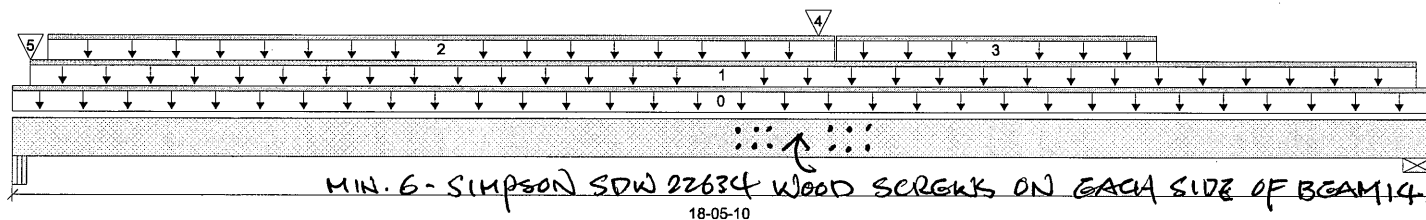

Bearing Supports

	Dim. (LxW)	Demand	Demand/Resistance Support	Demand/Resistance Member	Material
B1	Hanger 2" x 5-1/4"	9134 lbs	n/a	71.3%	HGUS5.50/10
B2	Wall/Plate 3-1/2" x 5-1/4"	8667 lbs	76.7%	38.7%	Spruce-Pine-Fir

Cautions

Header for the hanger HGUS5.50/10 is a Quadruple 1-3/4" x 11-7/8" LVL Beam.

Hanger model HGUS5.50/10 and seat length were input by the user. Nail one ply to another with 3 1/2" spiral nails @ 10" o.c., staggered in 2 rows



Total Horizontal Product Length = 18-05-10

Reaction Summary (Down / Uplift) (lbs)

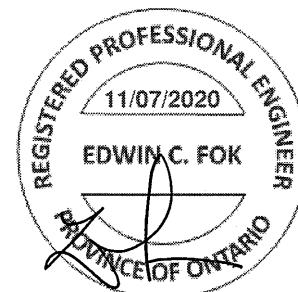
Bearing	Live	Dead	Snow	Wind
B1, 5-1/2"	2580 / 0	1929 / 0		
B2, 2-3/8"	2485 / 0	2045 / 0		

Load Summary

Tag	Description	Load Type	Ref.	Start	End	Loc.	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Tributary
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	18-05-10	Top		24			00-00-00
1	FC2 Floor Material	Unf. Lin. (lb/ft)	L	00-02-12	18-03-04	Top	20	10			n/a
2	FC2 Floor Material	Unf. Lin. (lb/ft)	L	00-05-08	10-08-08	Top	14	7			n/a
3	20(i21056)	Unf. Lin. (lb/ft)	L	10-08-12	14-10-12	Top		68			n/a
4	-	Conc. Pt. (lbs)	L	10-05-14	10-05-14	Top	4021	2638			n/a
5	18(i21054)	Conc. Pt. (lbs)	L	00-02-12	00-02-12	Top	536	319			n/a

Controls Summary

	Factored Demand	Factored Resistance	Demand/Resistance	Case	Location
Pos. Moment	45845 ft-lbs	73615 ft-lbs	62.3%	1	10-05-14
End Shear	6201 lbs	28927 lbs	21.4%	1	17-03-06
Total Load Deflection	L/270 (0.798")	n/a	88.9%	4	09-07-11
Live Load Deflection	L/475 (0.453")	n/a	75.7%	5	09-07-11
Max Defl.	0.798"	n/a	n/a	4	09-07-11
Span / Depth	18.1				



Bearing Supports

	Dim. (LxW)	Demand	Demand/Resistance Support	Demand/Resistance Member	Material
B1	Beam 5-1/2" x 7"	6281 lbs	26.5%	13.4%	Unspecified
B2	Wall/Plate 2-3/8" x 7"	6283 lbs	61.4%	31.0%	Spruce-Pine-Fir

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor : Normal Part code : Part 9

Nail one ply to another with

3 1/2" spiral nails @ 12"

o.c, staggered in 2 rows, plus 1/2" of boots,

NOTES & WASHERS @ 4" o.c.,

STAGGERED IN 2 ROWS

BC CALC® Member Report
 Build 7493

Dry | 1 span | No cant.

June 20, 2020 12:28:05

Job name:

File name: 4205 EL. A.mmdl

Address:

Description: Ground Floor - Supply/BOM\Flush Beams\B16(i28134)

City, Province, Postal Code:

Specifier:

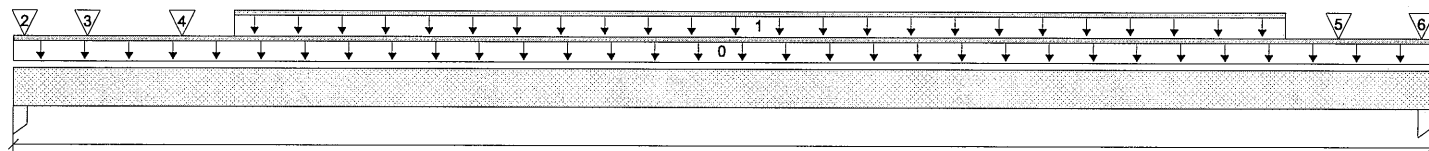
Customer:

Designer:

Code reports:

CCMC 12472-R

Company:



B1

13-05-14

B2

Total Horizontal Product Length = 13-05-14

Reaction Summary (Down / Uplift) (lbs)

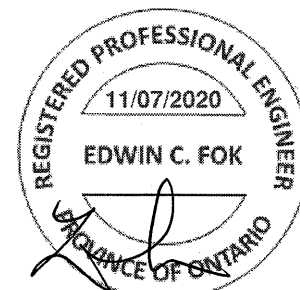
Bearing	Live	Dead	Snow	Wind
B1, 2-3/8"	985 / 0	524 / 0		
B2, 2-3/8"	984 / 0	525 / 0		

Load Summary

Tag	Description	Load Type	Ref.	Start	End	Loc.	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Tributary
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	13-05-14	Top		5			00-00-00
1	Smoothed Load	Unf. Lin. (lb/ft)	L	02-01-02	12-01-02	Top	146	73			n/a
2	J1(i28124)	Conc. Pt. (lbs)	L	00-01-04	00-01-04	Top	58	29			n/a
3	J1(i28121)	Conc. Pt. (lbs)	L	00-08-06	00-08-06	Top	109	54			n/a
4	J1(i28120)	Conc. Pt. (lbs)	L	01-07-02	01-07-02	Top	139	69			n/a
5	J1(i28109)	Conc. Pt. (lbs)	L	12-07-02	12-07-02	Top	131	66			n/a
6	J1(i28108)	Conc. Pt. (lbs)	L	13-04-10	13-04-10	Top	72	36			n/a

Controls Summary

	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	6909 ft-lbs	11610 ft-lbs	59.5%	1	06-07-02
End Shear	1940 lbs	5785 lbs	33.5%	1	12-06-00
Total Load Deflection	L/259 (0.612")	n/a	92.7%	4	06-10-02
Live Load Deflection	L/397 (0.4")	n/a	90.7%	5	06-10-02
Max Defl.	0.612"	n/a	n/a	4	06-10-02
Span / Depth	16.7				



Bearing Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1 Column	2-3/8" x 1-3/4"	2133 lbs	49.1%	42.1%	Unspecified
B2 Column	2-3/8" x 1-3/4"	2132 lbs	49.1%	42.0%	Unspecified

Notes

Design meets Code minimum (L/240) Total load deflection criteria.
 Design meets Code minimum (L/360) Live load deflection criteria.
 Calculations assume member is fully braced.
 Resistance Factor phi has been applied to all presented results per CSA O86.
 BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.
 Design based on Dry Service Condition.
 Importance Factor : Normal Part code : Part 9

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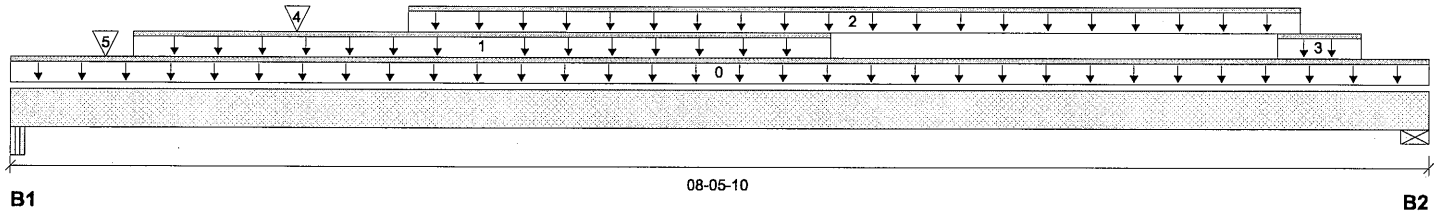
BC CALC® Member Report
 Build 7493
 Job name:
 Address:
 City, Province, Postal Code:
 Customer:
 Code reports:

Dry | 1 span | No cant.

June 20, 2020 14:58:19

File name: 4205 EL. A - OPT. 9' BSMT.mmdl
 Description: Ground Floor - Supply/BOM\Flush Beams\B17(i33656)
 Specifier:
 Designer:
 Company:

CCMC 12472-R



Total Horizontal Product Length = 08-05-10

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B1, 5-1/2"	345 / 0	410 / 0		
B2, 2-3/8"	315 / 0	301 / 0		

Load Summary

Tag	Description	Load Type	Ref.	Start	End	Loc.	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Tributary
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	08-05-10	Top		6			00-00-00
1	20(i21056)	Unf. Lin. (lb/ft)	L	00-08-12	04-10-12	Top		68			n/a
2	Smoothed Load	Unf. Lin. (lb/ft)	L	02-04-06	07-08-06	Top	93	46			n/a
3	21(i21057)	Unf. Lin. (lb/ft)	L	07-06-12	08-00-12	Top		68			n/a
4	J6(i33655)	Conc. Pt. (lbs)	L	01-08-06	01-08-06	Top	124	65			n/a
5	19(i21055)	Conc. Pt. (lbs)	L	00-06-12	00-06-12	Top	30	25			n/a

Controls Summary

	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	2051 ft-lbs	17696 ft-lbs	11.6%	1	04-04-06
End Shear	934 lbs	7232 lbs	12.9%	1	01-05-06
Total Load Deflection	L/999 (0.034")	n/a	n/a	4	04-03-06
Live Load Deflection	L/999 (0.017")	n/a	n/a	5	04-04-06
Max Defl.	0.034"	n/a	n/a	4	04-03-06
Span / Depth	8.0				



Bearing Supports

	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Beam 5-1/2" x 1-3/4"	1030 lbs	17.4%	8.8%	Unspecified
B2	Wall/Plate 2-3/8" x 1-3/4"	848 lbs	33.1%	16.7%	Spruce-Pine-Fir

Notes

Design meets Code minimum (L/240) Total load deflection criteria.
 Design meets Code minimum (L/360) Live load deflection criteria.
 Calculations assume member is fully braced.
 Resistance Factor phi has been applied to all presented results per CSA O86.
 BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.
 Design based on Dry Service Condition.
 Importance Factor : Normal Part code : Part 9

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BC CALC®, BC FRAMER®, AJS™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,

BC CALC® Member Report
 Build 7493

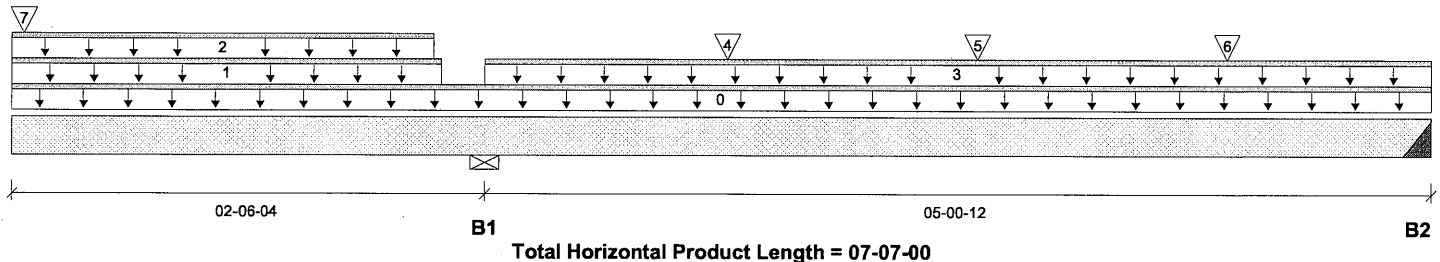
Dry | 2 spans | L cant.

June 20, 2020 15:14:15

Job name:
 Address:
 City, Province, Postal Code:
 Customer:
 Code reports:

CCMC 12472-R

File name: 4205 EL. B.mmdl
 Description: 2nd Floor - Supply/BOM\Flush Beams\B3b(i31139)
 Specifier:
 Designer:
 Company:



Reaction Summary (Down / Uplift) (lbs)

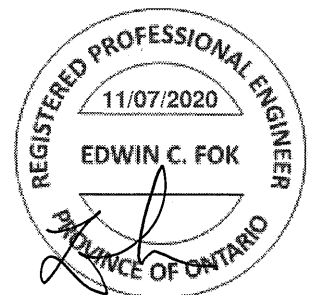
Bearing	Live	Dead	Snow	Wind
B1, 5-1/2"	1166 / 0	902 / 0		
B2, 2"	345 / 272	0 / 36		

Load Summary

Tag	Description	Load Type	Ref.	Start	End	Loc.	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Tributary
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	07-07-00	Top		6			00-00-00
1	FC1 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	02-03-08	Top	29	14			n/a
2	W5(i21018)	Unf. Lin. (lb/ft)	L	00-00-00	02-03-00	Top		26			n/a
3	FC1 Floor Material	Unf. Lin. (lb/ft)	L	02-06-04	07-07-00	Top	27	14			n/a
4	J6(i31592)	Conc. Pt. (lbs)	L	03-09-14	03-09-14	Top	177	89			n/a
5	J6(i31553)	Conc. Pt. (lbs)	L	05-01-14	05-01-14	Top	178	89			n/a
6	J6(i31395)	Conc. Pt. (lbs)	L	06-05-14	06-05-14	Top	167	83			n/a
7	-	Conc. Pt. (lbs)	L	00-00-12	00-00-12	Top	513	390			n/a

Controls Summary

	Factored Demand	Factored Resistance	Demand/Resistance	Case	Location
Pos. Moment	521 ft-lbs	17696 ft-lbs	2.9%	6	05-01-14
Neg. Moment	-3409 ft-lbs	-17696 ft-lbs	19.3%	1	02-06-04
End Shear	426 lbs	7232 lbs	5.9%	2	06-05-02
Cont. Shear	1420 lbs	7232 lbs	19.6%	1	01-03-10
Total Load Deflection	2xL/1998 (0.05")	n/a	n/a	9	00-00-00
Live Load Deflection	2xL/1998 (0.03")	n/a	n/a	12	00-00-00
Total Neg. Defl.	L/999 (-0.011")	n/a	n/a	9	04-06-06
Max Defl.	-0.011"	n/a	n/a	9	04-06-06
Span / Depth	5.0				



Bearing Supports	Dim. (LxW)	Demand	Demand/Resistance Support	Demand/Resistance Member	Material
B1	Wall/Plate 5-1/2" x 1-3/4"	2876 lbs	48.6%	24.5%	Spruce-Pine-Fir
B2	Hanger 2" x 1-3/4"	485 lbs	n/a	11.4%	HUS1.81/10
B2	Uplift	453 lbs			

Cautions

Uplift of 453 lbs found at bearing B2. (**HANGER**)

Header for the hanger HUS1.81/10 is a Single 1-3/4" x 11-7/8" LVL Beam.
 Hanger model HUS1.81/10 and seat length were input by the user. |

Long Cantilever: Sheathing required on bottom flange and adjacent back span or bracing designed by the design professional of record. Design professional of record must address uplift at supports.

Single 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP
2nd Floor - Supply/BOM\Flush Beams\B4b(i31111) (Flush Beam)

PASSED

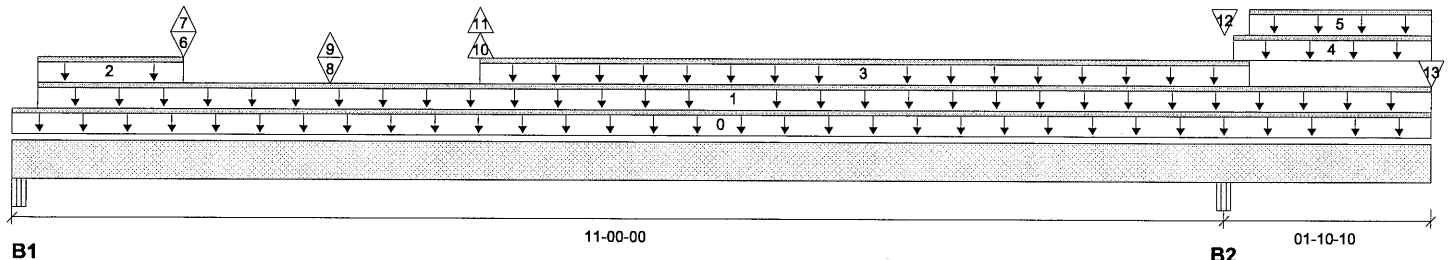
BC CALC® Member Report
 Build 7493
 Job name:
 Address:
 City, Province, Postal Code:
 Customer:
 Code reports:

Dry | 2 spans | R cant.

June 20, 2020 15:14:15

File name: 4205 EL. B.mmdl
 Description: 2nd Floor - Supply/BOM\Flush Beams\B4b(i31111)
 Specifier:
 Designer:
 Company:

CCMC 12472-R



Total Horizontal Product Length = 12-10-10

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B1, 5-1/2"	617 / 230	152 / 0	0 / 22	
B2, 5-1/2"	415 / 110	531 / 0	229 / 0	

Load Summary

Tag	Description	Load Type	Ref.	Start	End	Loc.	Live	Dead	Snow	Wind	Tributary
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	12-10-10	Top	1.00	0.65	1.00	1.15	00-00-00
1	FC1 Floor Material	Unf. Lin. (lb/ft)	L	00-02-12	12-10-10	Top	17	9			n/a
2	FC1 Floor Material	Unf. Lin. (lb/ft)	L	00-02-12	01-06-08	Top	38	19			n/a
3	FC1 Floor Material	Unf. Lin. (lb/ft)	L	04-02-14	11-02-12	Top	23	12			n/a
4	50(i28345)	Unf. Lin. (lb/ft)	L	11-01-00	12-10-10	Top		101			n/a
5	50(i28345)	Unf. Lin. (lb/ft)	L	11-02-12	12-10-10	Top		74	116		n/a
6	J5(i31082)	Conc. Pt. (lbs)	L	01-06-08	01-06-08	Top	114	42			n/a
7	J5(i31082)	Conc. Pt. (lbs)	L	01-06-08	01-06-08	Top	-31				n/a
8	J5(i31144)	Conc. Pt. (lbs)	L	02-10-08	02-10-08	Top	141	54			n/a
9	J5(i31144)	Conc. Pt. (lbs)	L	02-10-08	02-10-08	Top	-33				n/a
10	B3b(i31139)	Conc. Pt. (lbs)	L	04-02-14	04-02-14	Top	340	-36			n/a
11	B3b(i31139)	Conc. Pt. (lbs)	L	04-02-14	04-02-14	Top	-272				n/a
12	51(i28343)	Conc. Pt. (lbs)	L	11-00-00	11-00-00	Top		15			n/a
13	FC1 Floor Material	Conc. Pt. (lbs)	L	12-10-10	12-10-10	Top		10	15		n/a

Controls Summary

	Factored Demand	Factored Resistance	Demand/Resistance	Case	Location
Pos. Moment	2596 ft-lbs	17696 ft-lbs	14.7%	43	04-02-14
Neg. Moment	-1042 ft-lbs	-17696 ft-lbs	5.9%	24	04-02-14
End Shear	963 lbs	7232 lbs	13.3%	43	01-05-06
Cont. Shear	679 lbs	7232 lbs	9.4%	1	09-09-06
Total Load Deflection	L/999 (0.065")	n/a	n/a	102	05-04-06
Live Load Deflection	L/999 (0.059")	n/a	n/a	151	05-05-07
Total Neg. Defl.	2xL/1998 (-0.03")	n/a	n/a	102	12-10-10
Max Defl.	0.065"	n/a	n/a	102	05-04-06
Span / Depth	10.7				



Bearing Supports	Dim. (LxW)	Demand	Demand/Resistance Support	Demand/Resistance Member	Material
B1 Beam	5-1/2" x 1-3/4"	1115 lbs	15.1%	9.5%	Unspecified
B1 Uplift		230 lbs	(1-H3 Actual)		
B2 Beam	5-1/2" x 1-3/4"	1515 lbs	20.5%	12.9%	Unspecified

83020592

Single 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

2nd Floor - Supply/BOM\Flush Beams\B3c(i34744) (Flush Beam)

PASSED

BC CALC® Member Report
Build 7493

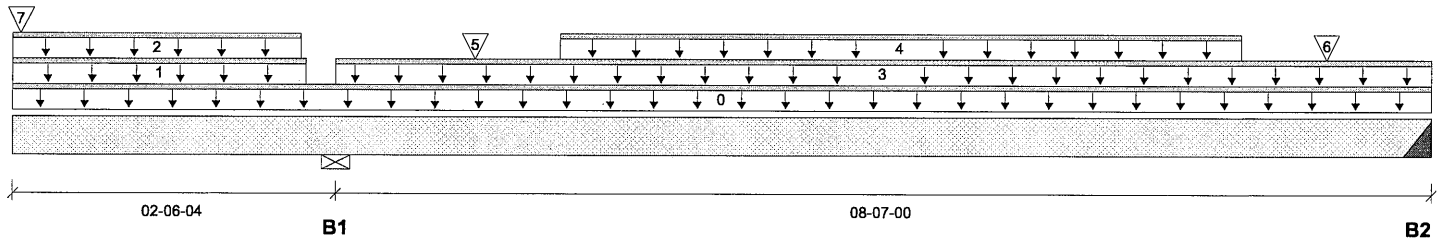
Dry | 2 spans | L cant.

June 22, 2020 08:32:00

Job name:
Address:
City, Province, Postal Code:
Customer:
Code reports:

CCMC 12472-R

File name: 4205 EL. C.mmdl
Description: 2nd Floor - Supply/BOM\Flush Beams\B3c(i34744)
Specifier:
Designer:
Company:



Total Horizontal Product Length = 11-01-04

Reaction Summary (Down / Uplift) (lbs)

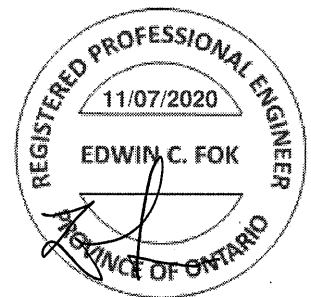
Bearing	Live	Dead	Snow	Wind
B1, 5-1/2"	1541 / 0	1062 / 0		
B2, 2"	847 / 159	317 / 0		

Load Summary

Tag	Description	Load Type	Ref.	Start	End	Loc.	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Tributary
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	11-01-04	Top		6			00-00-00
1	FC1 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	02-03-08	Top	29	14			n/a
2	W5(i21018)	Unf. Lin. (lb/ft)	L	00-00-00	02-03-00	Top		26			n/a
3	FC1 Floor Material	Unf. Lin. (lb/ft)	L	02-06-04	11-01-04	Top	27	14			n/a
4	Smoothed Load	Unf. Lin. (lb/ft)	L	04-03-06	09-07-06	Top	185	92			n/a
5	J5(i34832)	Conc. Pt. (lbs)	L	03-07-06	03-07-06	Top	227	113			n/a
6	J5(i34868)	Conc. Pt. (lbs)	L	10-03-06	10-03-06	Top	200	100			n/a
7	-	Conc. Pt. (lbs)	L	00-00-12	00-00-12	Top	513	390			n/a

Controls Summary

	Factored Demand	Factored Resistance	Demand/Resistance	Case	Location
Pos. Moment	3412 ft-lbs	17696 ft-lbs	19.3%	3	07-07-06
Neg. Moment	-3409 ft-lbs	-17696 ft-lbs	19.3%	1	02-06-04
End Shear	1448 lbs	7232 lbs	20.0%	3	09-11-06
Cont. Shear	1997 lbs	7232 lbs	27.6%	1	03-08-14
Total Load Deflection	L/999 (0.059")	n/a	n/a	10	06-11-06
Live Load Deflection	L/999 (0.05")	n/a	n/a	13	06-09-06
Total Neg. Defl.	2xL/1998 (-0.036")	n/a	n/a	10	00-00-00
Max Defl.	0.059"	n/a	n/a	10	06-11-06
Span / Depth	8.6				



Bearing Supports

	Dim. (LxW)	Demand	Demand/Resistance Support	Demand/Resistance Member	Material
B1	Wall/Plate 5-1/2" x 1-3/4"	3638 lbs	61.4%	31.0%	Spruce-Pine-Fir
B2	Hanger 2" x 1-3/4"	1667 lbs	n/a	39.0%	HUS1.81/10

Cautions

Header for the hanger HUS1.81/10 is a Single 1-3/4" x 11-7/8" LVL Beam.
Hanger model HUS1.81/10 and seat length were input by the user.

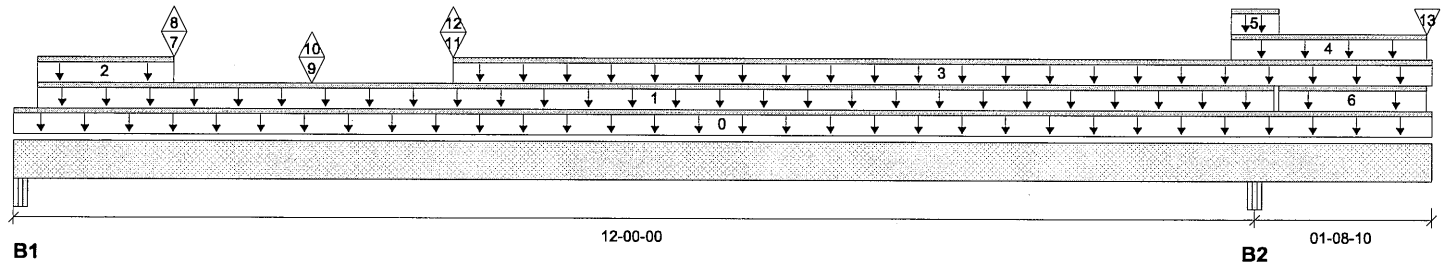
BC CALC® Member Report
 Build 7493
 Job name:
 Address:
 City, Province, Postal Code:
 Customer:
 Code reports:

Dry | 2 spans | R cant.

June 22, 2020 08:32:00

File name: 4205 EL. C.mmdl
 Description: 2nd Floor - Supply/BOM\Flush Beams\B4c(i34934)
 Specifier:
 Designer:
 Company:

CCMC 12472-R



Total Horizontal Product Length = 13-08-10

Reaction Summary (Down / Uplift) (lbs)

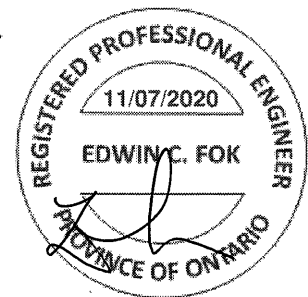
Bearing	Live	Dead	Snow	Wind
B1, 5-1/2"	1121 / 141	485 / 0	0 / 18	
B2, 5-1/2"	578 / 59	677 / 0	260 / 0	

Load Summary

Tag	Description	Load Type	Ref.	Start	End	Loc.	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Tributary
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	13-08-10	Top		6			00-00-00
1	FC1 Floor Material	Unf. Lin. (lb/ft)	L	00-02-12	12-02-03	Top	19	9			n/a
2	FC1 Floor Material	Unf. Lin. (lb/ft)	L	00-02-12	01-06-08	Top	29	14			n/a
3	FC1 Floor Material	Unf. Lin. (lb/ft)	L	04-02-14	13-08-10	Top	16	8			n/a
4	52(i32191)	Unf. Lin. (lb/ft)	L	11-09-04	13-08-00	Top		101			n/a
5	52(i32191)	Unf. Lin. (lb/ft)	L	11-09-04	12-02-12	Top		44	68		n/a
6	52(i32191)	Unf. Lin. (lb/ft)	L	12-02-12	13-08-00	Top		87	136		n/a
7	J4(i34725)	Conc. Pt. (lbs)	L	01-06-08	01-06-08	Top	208	95			n/a
8	J4(i34725)	Conc. Pt. (lbs)	L	01-06-08	01-06-08	Top	-18				n/a
9	J4(i34604)	Conc. Pt. (lbs)	L	02-10-08	02-10-08	Top	232	106			n/a
10	J4(i34604)	Conc. Pt. (lbs)	L	02-10-08	02-10-08	Top	-20				n/a
11	B3c(i34744)	Conc. Pt. (lbs)	L	04-02-14	04-02-14	Top	836	313			n/a
12	B3c(i34744)	Conc. Pt. (lbs)	L	04-02-14	04-02-14	Top	-160				n/a
13	52(i32191)	Conc. Pt. (lbs)	L	13-08-00	13-08-00	Top		18	14		n/a

Controls Summary

	Factored Demand	Factored Resistance	Demand/Resistance	Case	Location
Pos. Moment	6318 ft-lbs	17696 ft-lbs	35.7%	59	04-02-14
Neg. Moment	-731 ft-lbs	-17696 ft-lbs	4.1%	65	12-00-00
End Shear	2153 lbs	7232 lbs	29.8%	59	01-05-06
Cont. Shear	1105 lbs	7232 lbs	15.3%	21	10-09-06
Total Load Deflection	L/759 (0.184")	n/a	31.6%	138	05-08-01
Live Load Deflection	L/1051 (0.133")	n/a	34.3%	205	05-09-05
Total Neg. Defl.	2xL/1998 (-0.073")	n/a	n/a	138	13-08-10
Max Defl.	0.184"	n/a	n/a	138	05-08-01
Span / Depth	11.7				



Bearing Supports

	Dim. (LxW)	Demand	Demand/Resistance Support	Demand/Resistance Member	Material
B1	Beam 5-1/2" x 1-3/4"	2287 lbs	38.6%	19.5%	Unspecified
B2	Beam 5-1/2" x 1-3/4"	1972 lbs	33.3%	16.8%	Unspecified

Maximum Floor Spans – M4.1, L/360

Design Criteria

Spans:	Simple span
Loads:	Live load = 40 psf and dead load = 20 psf
Deflection limits:	L/360 under live load and L/240 under total load
Sheathing:	3/4 in. nailed-glued oriented strand board (OSB) sheathing



Maximum Floor Spans

Joist depth	Joist series	Bare				1/2 in. gypsum ceiling			
		On centre spacing				On centre spacing			
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
9-1/2"	NI-20	15'-11"	15'-0"	14'-6"	13'-5"	16'-5"	15'-5"	14'-6"	13'-5"
	NI-40x	17'-0"	16'-0"	15'-5"	14'-10"	17'-5"	16'-5"	15'-10"	14'-11"
	NI-60	17'-2"	16'-2"	15'-7"	14'-11"	17'-7"	16'-7"	16'-0"	15'-4"
	NI-80	18'-3"	17'-1"	16'-5"	15'-9"	18'-8"	17'-5"	16'-9"	16'-1"
11-7/8"	NI-20	17'-11"	16'-11"	16'-3"	15'-8"	18'-7"	17'-5"	16'-10"	16'-1"
	NI-40x	19'-4"	17'-11"	17'-3"	16'-7"	19'-11"	18'-6"	17'-9"	17'-0"
	NI-60	19'-7"	18'-2"	17'-6"	16'-9"	20'-2"	18'-9"	17'-11"	17'-2"
	NI-80	21'-1"	19'-6"	18'-6"	17'-7"	21'-7"	20'-0"	19'-0"	18'-0"
	NI-90	21'-6"	19'-10"	18'-11"	17'-11"	22'-0"	20'-4"	19'-5"	18'-4"
14"	NI-40x	21'-5"	19'-11"	18'-11"	18'-0"	22'-1"	20'-7"	19'-7"	18'-7"
	NI-60	21'-10"	20'-2"	19'-3"	18'-3"	22'-6"	20'-10"	19'-11"	18'-10"
	NI-80	23'-5"	21'-7"	20'-7"	19'-5"	24'-0"	22'-3"	21'-2"	20'-0"
	NI-90	23'-10"	22'-1"	21'-0"	19'-10"	24'-5"	22'-7"	21'-6"	20'-4"
16"	NI-60	23'-9"	22'-0"	21'-0"	19'-10"	24'-6"	22'-9"	21'-8"	20'-7"
	NI-80	25'-6"	23'-7"	22'-5"	21'-2"	26'-2"	24'-3"	23'-1"	21'-10"
	NI-90	26'-0"	24'-0"	22'-10"	21'-6"	26'-7"	24'-8"	23'-5"	22'-2"

Joist depth	Joist series	Mid-span blocking with 1x4 inch strap				Mid-span blocking and 1/2 in. gypsum ceiling			
		On centre spacing				On centre spacing			
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
9-1/2"	NI-20	17'-1"	15'-5"	14'-6"	13'-5"	17'-1"	15'-5"	14'-6"	13'-5"
	NI-40x	18'-8"	17'-6"	16'-7"	14'-11"	19'-2"	17'-8"	16'-7"	14'-11"
	NI-60	18'-11"	17'-8"	16'-10"	15'-7"	19'-5"	18'-0"	16'-10"	15'-7"
	NI-80	20'-3"	18'-10"	17'-11"	17'-2"	20'-8"	19'-3"	18'-4"	17'-5"
11-7/8"	NI-20	20'-3"	18'-8"	17'-6"	16'-1"	20'-7"	18'-8"	17'-6"	16'-1"
	NI-40x	21'-10"	20'-4"	19'-0"	17'-0"	22'-5"	20'-10"	19'-0"	17'-0"
	NI-60	22'-1"	20'-7"	19'-8"	18'-7"	22'-8"	21'-2"	20'-3"	18'-8"
	NI-80	23'-8"	22'-0"	20'-11"	19'-10"	24'-1"	22'-6"	21'-6"	20'-4"
	NI-90	24'-1"	22'-5"	21'-4"	20'-2"	24'-7"	22'-11"	21'-10"	20'-8"
14"	NI-40x	24'-5"	22'-9"	20'-11"	18'-8"	25'-1"	22'-11"	20'-11"	18'-8"
	NI-60	24'-10"	23'-2"	22'-1"	20'-10"	25'-6"	23'-10"	22'-9"	21'-4"
	NI-80	26'-6"	24'-8"	23'-6"	22'-2"	27'-1"	25'-3"	24'-1"	22'-9"
	NI-90	27'-0"	25'-1"	23'-11"	22'-7"	27'-6"	25'-8"	24'-6"	23'-2"
16"	NI-60	27'-3"	25'-5"	24'-3"	22'-11"	28'-0"	26'-2"	25'-0"	23'-1"
	NI-80	29'-1"	27'-1"	25'-9"	24'-4"	29'-8"	27'-9"	26'-5"	25'-0"
	NI-90	29'-7"	27'-6"	26'-2"	24'-9"	30'-2"	28'-2"	26'-10"	25'-5"

Notes:

- The tabulated clear spans are based on CSA O86-14 and NBC 2015, and are applicable to residential floor construction meeting the above design criteria.
- For multiple-span applications, the end spans shall be 40% or more of the adjacent span.
- Minimum bearing length shall be 1-3/4 inch for end bearings, and 3-1/2 inches for intermediate bearings.
- Bearing stiffeners are not required when I-joists are used in accordance with this table, except as required for hangers.
- Nordic I-joists are listed in CCMC Evaluation Report 13032-R and APA Product Report PR-L274C.

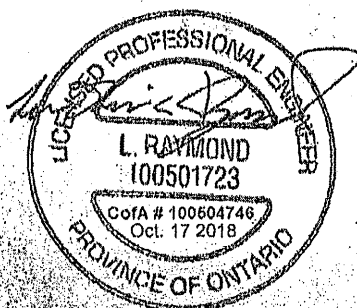
The construction details for residential designs are prone to changes.

Details released after April 2014 supersedes N-C301

Installation must comply with latest documentation on I-Joist and other Nordic products from the <http://nordic.ca/>

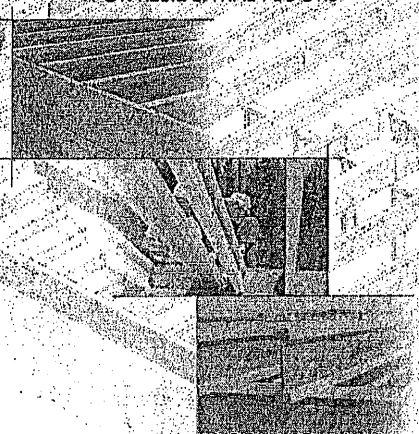
This document does not constitute a record of the structural integrity of the building nor suitability of the design assumptions made. Nordic Structures is responsible only for the structural adequacy of its component based on the design criteria and loadings shown on the calculation sheets.

Document prepared for the use of Stephanie Gon from Alpa, Ontario. (Nordic Request 1810-095)



N-C301/April 2014

NORDIC ENGINEERED WOOD INSTALLATION GUIDE FOR RESIDENTIAL FLOORS



Distributed by:



SAFETY AND CONSTRUCTION PRECAUTIONS

WARNING

I-Joists are not stable until completely installed, and will not carry any load until fully braced and sheathed.

Avoid Accidents by Following these Important Guidelines:



Do not walk on I-joists until fully braced and sheathed, or serious injuries can result.



Never stack building materials over unbraced I-joists. Once sheathed, do not over-stress I-joist with concentrated loads from building materials.

1. Brace and nail each I-joist as it is installed, using hangers, blocking panels, rim board, and/or cross-bracing at joist ends. When I-joists are applied continuous over interior supports and a load-bearing wall is planned at that location, blocking will be required at the interior support.
2. When the building is completed, the floor sheathing will provide lateral support for the top flanges of the I-joists. Until this sheathing is applied, temporary bracing, often called struts, or temporary sheathing must be applied.
 - Temporary bracing or struts must be 1x4 inch minimum, at least 8 feet long and spaced no more than 8 feet on centre, and must be secured with a minimum of two 2-1/2" nails fastened to the top surface of each I-joist. Nail the bracing to a lateral restraint at the end of each bay. Top ends of adjoining bracing over at least two I-joists.
 - Or, sheathing (temporary or permanent) can be nailed to the top flange of the first 4 feet of I-joists at the end of the bay.
3. For cantilevered I-joists, brace top and bottom flanges, and brace ends with closure panels, rim board, or cross-bracing.
4. Install and fully nail permanent sheathing to each I-joist before placing loads on the floor system. Then, stack building materials over beams or walls only.
5. Never install a damaged I-joist.

Improper storage or installation, failure to follow applicable building codes, failure to follow span ratings for Nordic I-joists, failure to follow allowable loads and locations, or failure to follow web stiffeners when required can result in serious accidents. Follow these installation guidelines carefully.

STORAGE AND HANDLING GUIDELINES

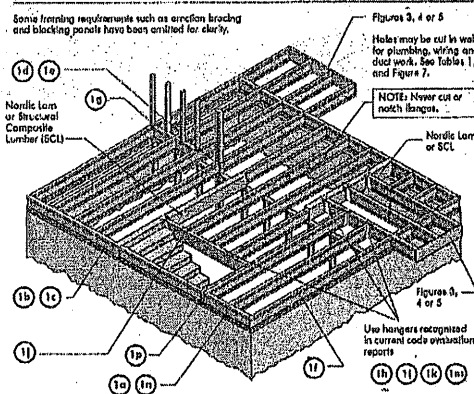
1. Bundle wrap can be slippery when wet. Avoid walking on wrapped bundles.
2. Store, stack, and handle I-joists vertically and level only.
3. Always stack and handle I-joists in the upright position only.
4. Do not store I-joists in direct contact with the ground and/or debris.
5. Protect I-joists from weather, and use spacers to separate bundles.
6. Bundled units should be kept intact until time of installation.
 - Pick I-joists in bundles as shipped by the supplier.
 - Orient the bundles so that the webs of the I-joists are vertical.
 - Pick the bundles at the 5th points, using a spreader bar if necessary.
8. Do not handle I-joists in a horizontal orientation.
9. NEVER USE AN TRY TO REPAIR A DAMAGED I-JOIST.



INSTALLING NORDIC I-JOISTS

1. Before laying out floor system components, verify that I-joist flange widths match hanger widths. If not, contact your supplier.
2. Except for cutting to length, I-joist flanges should never be cut, drilled, or notched.
3. Install I-joists so that top and bottom flanges are within 1/2 inch of true vertical alignment.
4. I-joists must be anchored securely to supports before floor sheathing is attached, and supports for multiple-span joists must be level.
5. Minimum bearing length: 1-3/4 inches for end bearings and 3-1/2 inches for intermediate bearings.
6. When using hangers, seat I-joists firmly in hanger bottoms to minimize settlement.
7. Leave a 1/16-inch gap between the I-joist and end a header.
8. Concentrated loads greater than those that can normally be expected in residential construction should only be applied to the top surface of the top flange. Normal concentrated loads include track lighting fixtures, audio equipment and security cameras. Never suspend loads or heavy loads from the I-joist's bottom flange. Whenever possible, suspend or concentrate loads from the top of the I-joist. Or, attach the load to blocking that has been securely fastened to the I-joist webs.
9. Never install I-joists where they will be permanently exposed to weather, or where they will remain in direct contact with concrete or masonry.
10. Restrain ends of floor joists to prevent rotation. Use rim board, rim joists or I-joist blocking panels.
11. For I-joists installed over and beneath bearing walls, use full depth blocking panels, rim board, or squash blocks (cripple members) to transfer gravity loads through the floor system to the wall or foundation below.
12. Due to shrinkage, common framing lumber set on edge may never be used as blocking or rim boards. I-joist blocking panels or other engineered wood products – such as rim board – must be cut to fit between the I-joists, and an I-joist-compatible depth selected.
13. Provide permanent lateral support of the bottom flange of all I-joists at interior supports of multiple-span joists. Similarly, support the bottom flange of all cantilevered I-joists at the end support next to the cantilever extension. In the completed structure, the gypsum wallboard ceiling provides this lateral support. Until the final finished ceiling is applied, temporary bracing or struts must be used.
14. If square-edge panels are used, edges must be supported between I-joists with 2x4 blocking. Use panels in blocking to minimize squeaks. Blocking is not required under structural finish flooring, such as wood strip flooring, or if a separate underlayment layer is installed.
15. Nail spacing: Space nails installed to the flange's top face in accordance with the applicable building code requirements or approved building plans.

FIGURE 1
TYPICAL NORDIC I-JOIST FLOOR FRAMING AND CONSTRUCTION DETAILS



All nails shown in the above details are assumed to be common wire nails unless otherwise noted. 3" (0.125" dia.) common spiral nails may be substituted for 2-1/2" (0.128" dia.) common wire nails. Framing lumber assumed to be Spruce-Pine-Fir No. 2 or better. Individual components not shown to scale for clarity.

1a

NI blocking panel

7-1/2" nails at 8" o.c. to top plate (when used for lateral shear transfer, nail to bearing plate with same nailing as required for blocking)

Attach I-joist to top plate per detail 1b

Blocking Panel or Rim Joist	Maximum Factored Uniform Vertical Load ¹ (q _u)
NI Joist	3,300

¹The uniform vertical load is limited to a joist depth of 16 inches or less and is based on standard term load duration. It shall not be used in the design of a bearing member, such as joist, header, or rafter. For concentrated vertical load transfer, see detail 1d.

1b

Rim board

One 2-1/2" face nail at each side of bearing

One 2-1/2" wire or spiral nail at top and bottom flange

Attach rim board to top plate using 2-1/2" wire or spiral nail at 8" o.c.

To avoid splitting flange, start nails at least 1-1/2" from end of I-joist. Nails may be driven at an angle to avoid splitting of bearing plate.

Minimum bearing length shall be 1-3/4" for the end bearings, and 3-1/2" for the intermediate bearings when applicable.

Blocking Panel or Rim Joist	Maximum Factored Uniform Vertical Load ¹ (q _u)
1-1/2" Rim Board Plus	0,020

¹The uniform vertical load is limited to a rim board depth of 16 inches or less and is based on standard term load duration. It shall not be used in the design of a bearing member, such as joist, header, or rafter. For concentrated vertical load transfer, see detail 1d.

1c

NI rim joist per detail 1a

Attach rim joist to floor joist with one nail at top and bottom. Nail must provide 1 inch minimum penetration into floor joist. Toe-nailing may be used.

Attach I-joist per detail 1b

Minimum 1-3/4" bearing required

1d

NI or rim board blocking panel per detail 1a

1-1/2" x 4" squash blocks

Squash block

Pair of Squash Blocks	Maximum Factored Vertical per Pair of Squash Blocks (q _u)
2x Lumber	5,500
1-1/2" Rim Board Plus	4,500

Provide lateral bracing per detail 1a, 1b, or 1c

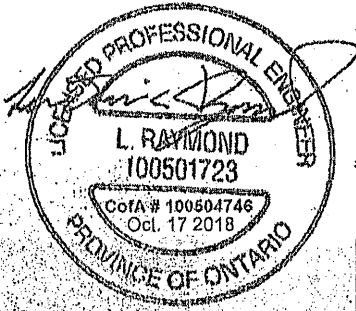
The construction details for residential designs are prone to changes.

Details released after April 2014 supersedes N-C301

Installation must comply with latest documentation on I-Joist and other Nordic products from the <http://nordic.ca/>

This document does not constitute a record of the structural integrity of the building nor suitability of the design assumptions made. Nordic Structures is responsible only for the structural adequacy of its component based on the design criteria and loadings shown on the calculation sheets.

Document prepared for the use of Stephanie Gon from Alpa, Ontario. (Nordic Request 1810-095)



N-C301/April 2014

MAXIMUM FLOOR SPANS

- Maximum clear spans applicable to single-span or multiple-span residential floor construction with a design live load of 40 psf and dead load of 15 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration and a live load deflection limit of L/480. For multiple-span applications, the end spans shall be 40% or more of the adjacent spans.
- Spans are based on a composite floor with glued-sheathed oriented strand board (OSB) sheathing with a minimum thickness of 5/8 inch for a joist spacing of 19.2 inches or less, or 3/4 inch for joist spacing of 24 inches. Adhesive shall meet the requirements given in CCB-7.1.24 Standard. No concrete topping or bridging element was assumed. Increased spans may be achieved with the use of gypsum and/or a row of blocking at mid-span.
- Minimum bearing length shall be 1-3/4 inches for the end bearings, and 2-1/2 inches for the intermediate bearings.
- Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.
- This span chart is based on uniform loads. For applications with other than uniform loads, an engineering analysis may be required based on the use of the design properties.
- Tables are based on Unit Stress Design per CAN/CSA C84-09 Standard, and NBC 2101.
- SI units conversions: 1 inch = 25.4 mm
1 foot = 0.305 m

MAXIMUM FLOOR SPANS FOR NORDIC I-JOISTS SIMPLE AND MULTIPLE SPANS

Span	2x6	2x8	2x10	2x12	2x14	2x16	2x18	2x20	2x22	2x24	2x26	2x28	2x30	2x32	2x34	2x36	2x38	2x40	2x42	2x44	2x46	2x48	2x50	2x52	2x54	2x56	2x58	2x60	2x62	2x64	2x66	2x68	2x70	2x72	2x74	2x76	2x78	2x80	2x82	2x84	2x86	2x88	2x90	2x92	2x94	2x96	2x98	2x100																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																							
1/2"	N-10	17-1	14-2	12-0	10-5	9-2	8-0	7-0	6-0	5-0	4-0	3-0	2-0	1-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-0	0-

Details released after April 2014
supersedes N-C301

Installation must comply with latest documentation on I-Joist and other Nordic products from the <http://nordic.ca/>

This document does not constitute a record of the structural integrity of the building nor suitability of the design assumptions made. Nordic Structures is responsible only for the structural adequacy of its component based on the design criteria and loadings shown on the calculation sheets.

Document prepared for the use of
Stephanie Gon from Alpa, Ontario.
(Nordic Request 1810-095)



CANTILEVER DETAILS FOR BALCONIES (NO WALL LOAD)

Cantilever extension supporting uniform floor loads only
 Rim board or wood structural panel closure; attach per detail 1b
 Attach 1-pile to plate of all supports per detail 1b
 1-pile, or rim board
 3-1/2" min. bearing required
 1/2" x 1/2" x 1/2" min. bearing required

CAUTION: Cantilevers formed this way must be carefully detailed to prevent moisture transfer from the structure and potential decay of untreated joist extensions.

Note: This detail is applicable to cantilevers supporting a maximum specified uniform live load of 60 psf.

Full depth backer block with 1/8" gap between black and top flange of I-beam. See detail 1b. Nail with 2 rows of 3" nails of 6" o.c. and clinch.

2d6 min. Nail to backer block and joist with 2 rows of 3" nails of 6" o.c. and clinch. (Cantilever nails may be used to attach backer block if length of nail is sufficient to allow clinching.)

Cantilever extension supporting uniform floor loads only

Lumber or wood structural panel closure

Note: This detail is applicable to cantilevers supporting a maximum specified uniform live load of 60 psf

3 1/2" min. bearing required

I-beam, or rim board

Attach I-beams to plate or all supports per detail 1b

1/2" x 4" x 1/2" min. plate or 1/2" x 4" x 1/2" min. plate or 1/2" x 4" x 1/2" min. plate

1. The first step in the process is to identify the problem or issue that needs to be addressed. This involves gathering information and understanding the context of the problem.

Rim band or wood structural
 steel plate 1/2" minimum
 thickness attach per detail 1b
 6°
 2 1/2" nails
 3 1/2" min. blocking post
 Attach 1/2" to plate
 per detail 1b
 1/2" blocking post or rim band blocking
 attach per detail 1g

- Use same installation as Method 1 but reinforce both sides of I-joint with sheathing.
- Use nailing pattern shown for Method 1 with opposite face nailing offset by 3".

Note: Canadian softwood plywood sheathing or equivalent (minimum thickness 3/4") required on both sides of I-joint. Attach the full height of the joint. Nail with 2-1/2" nails at 6" o.c., top and bottom flanges. I-joint with glue grain horizontal. Attach I-joint to plate on all supports per detail 1b. Verify reinforced I-joint capacity.

Diagram illustrating the installation of a curb on a wall. The curb is shown being attached to the wall and the roof deck. The curb is a metal channel with a gasket and fasteners. The diagram shows the curb being attached to a wall and the roof deck. Labels indicate the curb is to be attached to the wall and the roof deck, and that the curb is to be attached to the wall and the roof deck.

Labels in the diagram include:

- Rim board, or wood structural panel closure (3/4" minimum thickness) attach per detail 1b
- blocking, attach per detail 1g
- Face nail two rows of 3" nails in 12" o.c. along the curb
- Liquid web and the fiber fabric to other 1/4" o.c. Face nail from opposite face by 3"
- Check if available (four nails per foot required, except two nails per foot required if checked)
- Attach 1/4" o.c. top plate of all supports per detail 1b
- min. flashing required
- Block 1-pair together with fiber fabric for the full length of the min. clearance
- For 1-pair lengths greater than 3 inches place an additional row of 3" nails along the

See table below for HL reinforcement requirements at condition.

Roof truss spans

2'-0" maximum overleaf

Cinder truss

Roof truss span

Jack trusses

2'-0" maximum overleaf

Reinforcing Bars to be placed in the concrete floor joists the HL reinforcement requirements for a span of 25 ft. shall be permitted to be used.

[illegible]

1. N = the minimum requirement:
 - a. 1 in. reinforced with 3/16" wood structural panel on one side only.
 - b. 2 in. reinforced with 3/16" wood structural panel on both sides, or double 1/4 in. plywood, 1/2 in. gypsum board or equivalent spanning the full length of the wall.
 - c. 2 in. dry board, 3/4 in. plywood panel, and 50 lb/yd² wall load. Wall load is based on 20 lb/yd² maximum with 2 in. or over 2 in. spacing.
2. For horizontal openings, 6 in. (10 ft) 20 lb/yd² openings spaced less than 20 ft on, additional panel thickness for openings: 1/4 in. for each 10 ft.
3. Table applies to holes 1/2 in. to 20 ft. If the area the floor space occupied for a design hole may be required.
4. For vertical openings, 2 in. (10 ft) 20 lb/yd² openings spaced less than 20 ft on, and a 2 in. board deflection limit of 1/4 in. (10 ft) or, requirements for lesser spacing.
5. For conventional roof construction using a ridge beam, the roof floor spans between the ridge beam and the ridge beam.
6. When the hole is formed using a ridge beam, the roof floor spans is required to be the same as the spacing with the hole in the beam.
7. Combined walls supporting light truss or roof layout may require additional reinforcement.

Provide full depth blocking between joists over supports (not shown).

Note: Canadian selfweld plywood sheathing or equivalent (minimum thickness 3/8" ECR required on sides of joist. Dashed shall match the full depth of the joist. Nail with 2 1/2" nails @ 6" o.c., top and bottom flanges. Install with faces joist horizontal. Attach block to provide all supports per detail 11. Verify reinforced slab capacity.

1 1/2" min.

3 1/2" min.

Nail reinforcement to top and bottom joint flanges with 2 1/2" nails @ 6" o.c. Fasten against blocking 3" when using reinforcement on both sides of joist.

1 1/2" min.

3 1/2" min.

Vertical: solid steel blocks (2x2 3-4 ft No. 2 or better) nailed through joint web and web of girder using 2 1/2" nails. Alternate for opposite side.

Nails: toe-roll at top and bottom flanges.

Hanger may be used in lieu of solid steel blocks.

Notes:

- Verify girder joint capacity if the block span exceeds the joint spacing.
- Anchor double-joint per detail 1p, if required.

See table below for NI reinforcement requirements of roof truss.

Roof truss span

2'-0" maximum overhang

Girder truss span

2'-0" maximum overhang

13'-0" maximum

Jack trusses

Trusses running parallel to the concrete floor joist. The 1-ft flat reinforcement requirements for a span of 26 ft. shall be permitted to be used.

		RMS SAUNG MYINT											
		1950-1951						1951-1952					
		1	2	3	4	5	6	1	2	3	4	5	6
2-1/2	26	1	X	X	X	X	X	2	X	X	X	X	X
	27	1	X	X	X	X	X	2	X	X	X	X	X
	28	1	X	X	X	X	X	2	X	X	X	X	X
	29	2	X	X	X	X	X	3	X	X	X	X	X
	30	2	X	X	X	X	X	3	X	X	X	X	X
1 1/2	26	X	X	2	X	X	X	X	X	X	X	X	X
	27	X	X	2	X	X	X	X	X	X	X	X	X
	28	X	X	2	X	X	X	X	X	X	X	X	X
	29	X	X	2	X	X	X	X	X	X	X	X	X
	30	X	X	2	X	X	X	X	X	X	X	X	X
1	26	N	N	X	X	X	X	X	X	X	X	X	X
	27	2	2	X	X	X	X	1	1	X	X	X	X
	28	1	1	X	X	X	X	1	1	X	X	X	X
	29	1	1	X	X	X	X	1	2	X	X	X	X
	30	1	1	X	X	X	X	1	2	X	X	X	X
1/2	26	1	1	X	X	X	X	2	2	X	X	X	X
	27	1	1	X	X	X	X	2	2	X	X	X	X
	28	1	1	X	X	X	X	2	2	X	X	X	X
	29	1	1	X	X	X	X	2	2	X	X	X	X
	30	1	1	X	X	X	X	2	2	X	X	X	X

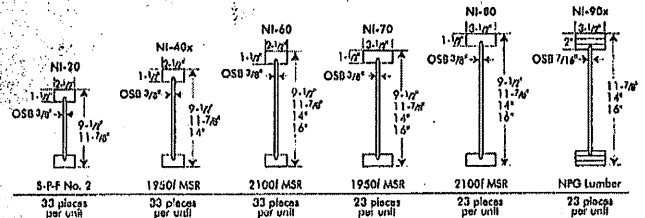
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CONSTRUCTION DETAILS FOR RESIDENTIAL FLOORS

N-C303 / September 2013



Refer to the Installation Guide for Residential Floors for additional information.
CCMC EVALUATION REPORT 13032-R



WEB HOLE SPECIFICATIONS

RULES FOR CUTTING HOLES AND DUCT CHASE OPENINGS:

- The distance between the inside edge of the support and the centreline of any hole or duct chase opening shall be in compliance with the requirements of Table 1 or 2, respectively.
- I-joist top and bottom flanges must NEVER be cut, notched, or otherwise modified.
- Whenever possible, field-cut holes should be centred on the middle of the web.
- The maximum size hole or the maximum depth of a duct chase opening that can be cut into an I-joist web shall equal the clear distance between the flanges of the I-joist minus 1/4 inch. A minimum of 1/8 inch should always be maintained between the top or bottom of the hole or opening and the adjacent I-joist flange.

- The sides of square holes or longest sides of rectangular holes should not exceed 3/4 of the diameter of the maximum round hole permitted at that location.
- Where more than one hole is necessary, the distance between adjacent hole edges shall exceed twice the diameter of the largest round hole or twice the size of the largest square hole (or twice the length of the longest side of the largest rectangular hole or duct chase opening) and each hole and duct chase opening shall be sized and located in compliance with the requirements of Tables 1 and 2, respectively.
- A knockout is not considered a hole, may be utilized anywhere it occurs, and may be ignored for purposes of calculating minimum distances between holes and/or duct chase openings.
- Holes measuring 1-1/2 inches or smaller are permitted anywhere in a cantilevered section of a joist. Holes of greater size may be permitted subject to verification.

- A 1-1/2 inch hole or smaller can be placed anywhere in the web provided that it meets the requirements of rule number 6 above.
- All holes and duct chase openings shall be cut in a workmanlike manner in accordance with the restrictions listed above and as illustrated in Figure 7.
- Limit three maximum size holes per span, of which one may be a duct chase opening.
- A group of round holes at approximately the same location shall be permitted if they meet the requirements for a single round hole circumscribed around them.

TABLE 1
LOCATION OF CIRCULAR HOLES IN JOIST WEBS

Simple or Multiple Span for Dead Loads up to 15 psf and Live Loads up to 40 psf

Joist Depth	Joist Series	Minimum Distance from Inside Face of Any Support to Centre of Hole (ft. - in.)												
		2	3	4	5	6	6-1/4	7	8	8-5/8	9	10	10-3/4	11
9-1/2"	NI-20	0-7"	1-4"	2-10"	4-3"	5-0"	6-0"	6-0"	...	...	...	...	...	...
	NI-40x	0-7"	1-4"	3-0"	4-4"	6-0"	6-4"	...	...	...	...	...	...	...
	NI-60	1-3"	2-6"	4-0"	5-4"	7-0"	7-5"	...	...	...	...	...	...	...
	NI-70	2-0"	3-4"	4-8"	6-3"	8-0"	8-4"	...	...	...	...	...	...	...
	NI-80	2-3"	3-6"	5-0"	6-5"	8-2"	8-8"	...	...	...	...	...	...	...
11-7/8"	NI-20	0-7"	0-8"	1-0"	2-4"	3-8"	4-0"	5-0"	6-6"	7-9"	...	...	...	...
	NI-40x	0-7"	0-8"	1-0"	2-4"	4-0"	4-4"	5-5"	7-0"	8-4"	...	...	...	...
	NI-60	0-7"	1-8"	2-0"	4-3"	6-9"	6-0"	7-5"	8-10"	10-0"	...	...	...	...
	NI-70	1-2"	2-0"	4-0"	5-4"	6-9"	7-5"	9-4"	10-0"	11-2"	...	...	...	...
	NI-80	1-6"	2-10"	4-2"	5-6"	7-0"	7-5"	8-6"	10-3"	11-4"	...	...	...	...
14"	NI-20	0-7"	0-8"	0-9"	2-5"	4-4"	4-9"	6-3"	...	...	...	...	...	...
	NI-40x	0-7"	0-8"	0-8"	1-0"	2-4"	2-9"	3-9"	5-2"	6-0"	6-6"	8-3"	10-2"	...
	NI-60	0-7"	0-8"	1-8"	3-0"	4-5"	5-10"	6-5"	7-3"	8-0"	9-9"	10-4"	12-0"	13-5"
	NI-70	0-8"	1-10"	3-0"	4-5"	5-10"	6-5"	7-3"	8-0"	9-9"	10-4"	12-0"	13-5"	...
	NI-80	0-10"	2-0"	3-4"	4-9"	6-2"	6-5"	7-6"	9-0"	10-0"	10-8"	12-4"	13-9"	...
16"	NI-20	0-7"	0-8"	0-8"	1-0"	2-10"	3-2"	4-2"	5-6"	6-4"	7-0"	8-5"	9-8"	10-2"
	NI-40x	0-7"	1-0"	2-3"	3-6"	4-10"	5-3"	6-3"	7-8"	8-4"	9-9"	10-8"	12-0"	13-9"
	NI-60	0-7"	1-0"	2-3"	3-6"	4-10"	5-3"	6-3"	7-8"	8-4"	9-9"	10-8"	12-0"	13-9"
	NI-70	0-7"	1-0"	2-3"	3-6"	4-10"	5-3"	6-3"	7-8"	8-4"	9-9"	10-8"	12-0"	13-9"
	NI-80	0-7"	1-0"	2-3"	3-6"	4-10"	5-3"	6-3"	7-8"	8-4"	9-9"	10-8"	12-0"	13-9"

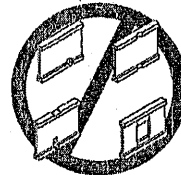
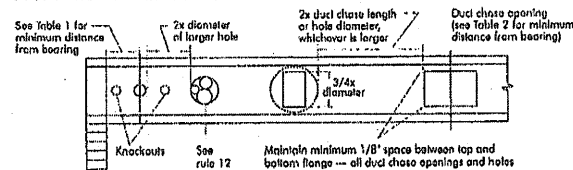
- Above table may be used for I-joist spacing of 24 inches on centre or less.
- Hole location distance is measured from inside face of support to centre of hole.
- Distances in this chart are based on uniformly loaded joists.
- The above table is based on the I-joists being used of their maximum spans. The minimum distance as given above may be reduced for shorter spans; contact your local distributor.

TABLE 2
DUCT CHASE OPENING SIZES AND LOCATIONS

Simple Span Only

Joist Depth	Joist Series	Minimum Distance from Inside Face of Supports to Centre of Opening (ft. - in.)												
		8	10	12	14	16	18	20	22	24	26	28	30	32
9-1/2"	NI-20	4-1"	4-5"	4-10"	5-4"	5-8"	6-1"	6-6"	7-1"	7-5"	...	...	...	...
	NI-40x	5-3"	5-8"	6-0"	6-5"	6-10"	7-3"	7-8"	8-2"	8-6"	...	...	...	...
	NI-60	5-4"	5-9"	6-2"	6-7"	7-1"	7-5"	8-0"	8-4"	8-9"	...	...	...	...
	NI-70	5-1"	5-5"	5-10"	6-3"	6-7"	7-1"	7-6"	8-1"	8-4"	...	...	...	...
	NI-80	5-3"	5-8"	6-0"	6-5"	6-10"	7-3"	7-8"	8-2"	8-6"	...	...	...	...
11-7/8"	NI-20	5-9"	6-2"	6-6"	7-1"	7-5"	7-9"	8-3"	8-7"	9-1"	...	...	...	...
	NI-40x	6-8"	7-2"	7-6"	8-1"	8-5"	9-1"	9-5"	10-1"	10-5"	...	...	...	...
	NI-60	7-3"	7-8"	8-0"	8-5"	9-0"	9-3"	9-9"	10-3"	10-8"	...	...	...	...
	NI-70	7-1"	7-6"	8-0"	8-5"	9-0"	9-3"	9-9"	10-3"	10-8"	...	...	...	...
	NI-80	7-2"	7-7"	8-0"	8-5"	9-0"	9-3"	9-9"	10-3"	10-8"	...	...	...	...
14"	NI-20	8-1"	8-5"	9-0"	9-4"	10-1"	10-5"	11-2"	11-6"	12-0"	12-4"	12-8"	13-2"	13-6"
	NI-40x	8-1"	8-5"	9-0"	9-4"	10-1"	10-5"	11-2"	11-6"	12-0"	12-4"	12-8"	13-2"	13-6"
	NI-60	8-9"	9-3"	9-8"	10-1"	10-6"	11-1"	11-6"	12-1"	12-6"	13-1"	13-6"	14-1"	14-6"
	NI-70	8-9"	9-3"	9-8"	10-1"	10-6"	11-1"	11-6"	12-1"	12-6"	13-1"	13-6"	14-1"	14-6"
	NI-80	9-0"	9-4"	9-9"	10-1"	10-7"	11-1"	11-6"	12-1"	12-6"	13-1"	13-6"	14-1"	14-6"
16"	NI-20	10-3"	10-8"	11-2"	11-6"	12-1"	12-6"	13-1"	13-6"	14-1"	14-6"	15-1"	15-6"	16-0"
	NI-40x	10-3"	10-8"	11-2"	11-6"	12-1"	12-6"	13-1"	13-6"	14-1"	14-6"	15-1"	15-6"	16-0"
	NI-60	10-1"	10-5"	11-0"	11-4"	11-9"	12-4"	12-9"	13-4"	13-9"	14-4"	14-9"	15-4"	15-9"
	NI-70	10-1"	10-5"	11-0"	11-4"	11-9"	12-4"	12-9"	13-4"	13-9"	14-4"	14-9"	15-4"	15-9"
	NI-80	10-4"	10-9"	11-3"	11-8"	12-3"	12-8"	13-3"	13-8"	14-3"	14-8"	15-3"	15-8"	16-3"

- Above table may be used for I-joist spacing of 24 inches on centre or less.
- Duct chase opening location distance is measured from inside face of support to centre of opening.
- The above table is based on simple-span joists only. For other applications, contact your local distributor.
- Distances are based on uniformly loaded floor joists that meet the span requirements for a design live load of 40 psf and dead load of 15 psf, and a live load deflection limit of L/480.
- The above table is based on the I-joists being used of their maximum spans. The minimum distance as given above may be reduced for shorter spans; contact your local distributor.

FIGURE 7
FIELD-CUT HOLE LOCATOR

Knockouts are pre-cored holes provided for the contractor's convenience to install electrical or small plumbing lines. They are 1-1/2 inches in diameter, and are spaced 15 inches on centre along the length of the joist. Where possible, it is preferable to use knockouts instead of field-cut holes.

Never drill, cut or notch the flange, or over-cut the web.

Holes in webs should be cut with a sharp saw.

For rectangular holes, avoid over-cutting the corners, as this can cause unnecessary stress concentrations. Slightly rounding the corners is recommended. Starting the rectangular hole by drilling a 1-inch diameter hole in each of the four corners and then making the cuts between the holes is another good method to minimize damage to the joist.

SAFETY AND CONSTRUCTION PRECAUTIONS



Do not walk on I-joist until fully fastened and braced, or serious injury can result.



Never stack building materials over unshathed I-joist. Once shathed, do not over-stress I-joists with concentrated loads from building materials.

WARNING: I-joists are not stable until completely installed, and will not carry any load until fully braced and sheathed.

AVOID ACCIDENTS BY FOLLOWING THESE IMPORTANT GUIDELINES:

- Brace and nail each I-joist as it is installed, using hangers, blocking panels, rim board, and/or cross-bracing at joist ends. When I-joists are applied continuous over interior supports and a load-bearing wall is planned at that location, blocking will be required at the interior support.
- When the building is completed, the floor sheathing will provide lateral support for the top flanges of the I-joists. Until this sheathing is applied, temporary bracing, often called struts, or temporary sheathing must be applied to prevent I-joist rollover or buckling.
 - Temporary bracing or struts must be 1x4 inch minimum, at least 8 feet long and spaced no more than 8 feet on centre, and must be secured with a minimum of two 2-1/2" nails fastened to the top surface of each I-joist. Nail the bracing to a lateral restraint at the end of each bay. Lap ends of adjoining bracing over at least two I-joists.
 - Or, sheathing (temporary or permanent) can be nailed to the top flange of the first 4 feet of I-joists at the end of the bay.
- For cantilevered I-joists, brace top and bottom flanges, and brace ends with closure panels, rim board, or cross-bracing.
- Install and fully nail permanent sheathing to each I-joist before placing loads on the floor system. Then, stack building materials over beams or walls only.
- Never install a damaged I-joist.

Improper storage or installation, failure to follow applicable building codes, failure to follow span ratings for Nordic I-joists, failure to follow allowable hole sizes and locations, or failure to use web stiffeners when required can result in serious accidents. Follow these installation guidelines carefully.



PRODUCT WARRANTY

Chambers Construction guarantees that, in accordance with our specifications, Nordic products are free from manufacturing defects in material and workmanship.

Furthermore, Chambers Construction warrants that our products, when utilized in accordance with our handling and installation instructions, will meet or exceed our specifications for the lifetime of the structure.

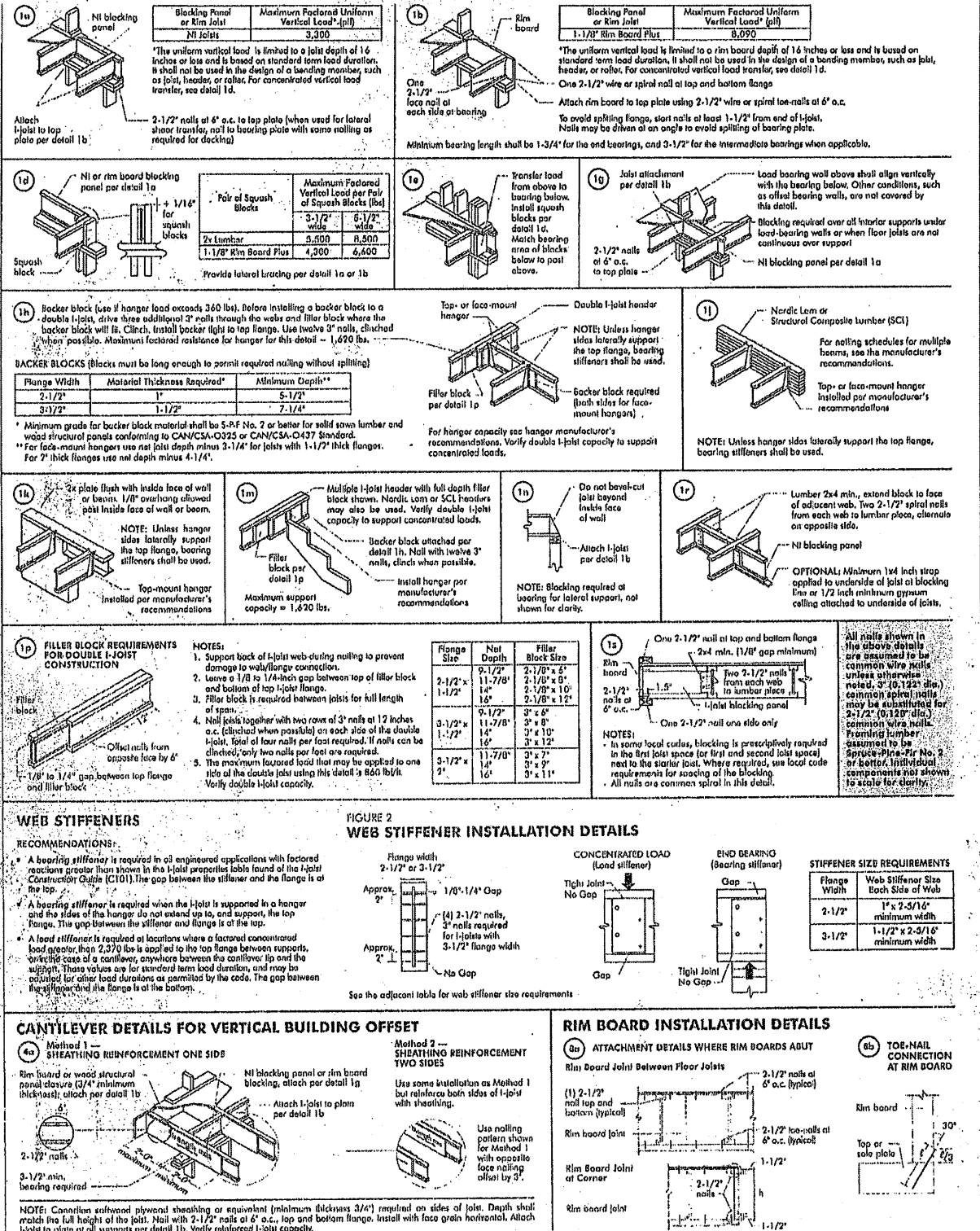
The construction details for residential designs are prone to changes.

Details released after September 2013 supersedes N-303

Installation must comply with latest documentation on I-Joist and other Nordic products from the <http://nordic.ca/>

This document does not constitute a record of the structural integrity of the building nor suitability of the design assumptions made. Nordic Structures is responsible only for the structural adequacy of its component based on the design criteria and loadings shown on the calculation sheets.

Document prepared for the use of Stephanie Gon from Alpa, Ontario. (Nordic Request 1810-095)



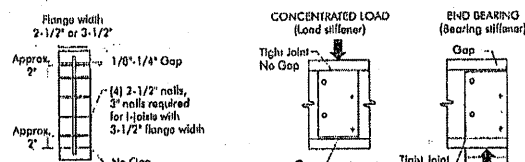
WEB STIFFENERS

RECOMMENDATIONS:

- A bearing stiffener is required in all engineered applications with factored reactions greater than shown in the I-joist properties table found in the Joist Construction Guide (C101). The gap between the stiffener and the flange is at the top.
- A bearing stiffener is required when the I-joist is supported in a hanger and the sides of the hanger do not extend up to, and support, the top flange. The gap between the stiffener and flange is at the top.
- A load stiffener is required at locations where a factored concentrated load greater than 2,370 lbs is applied to the top flange between supports, whether in the case of a cantilever, anywhere between the cantilever tip and the support. These values are for standard term load duration, and may be adjusted for other load durations as permitted by the code. The gap between the stiffener and the flange is at the bottom.

FIGURE 2

WEB STIFFENER INSTALLATION DETAILS

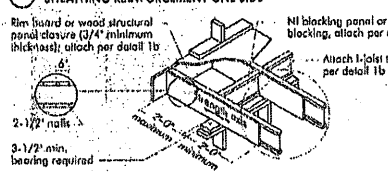


STIFFENER SIZE REQUIREMENTS

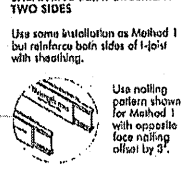
Flange Width	Web Stiffener Size Each Side of Web
2-1/2"	1" x 2-5/16" minimum width
3-1/2"	1-1/2" x 2-5/16" minimum width

CANTILEVER DETAILS FOR VERTICAL BUILDING OFFSET

Method 1 — SHEATHING REINFORCEMENT ONE SIDE



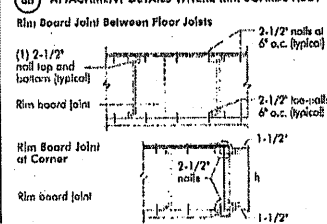
Method 2 — SHEATHING REINFORCEMENT TWO SIDES



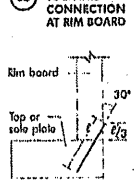
NOTE: Cantilever reinforced plywood sheathing or equivalent (minimum thickness 3/4") required on sides of joist. Depth shall reach the full height of the joist. Nail with 2-1/2" nails at 6" o.c., top and bottom flange. Install with face grain horizontal. Attach I-joist to plate at all supports per detail 1b. Verify reinforced I-joist capacity.

RIM BOARD INSTALLATION DETAILS

9a ATTACHMENT DETAILS WHERE RIM BOARDS ABOUT



9b TOE-NAIL CONNECTION AT RIM BOARD



The construction details for residential designs are prone to changes.

Details released after September 2013 supersedes N-303

Installation must comply with latest documentation on I-Joist and other Nordic products from the <http://nordic.ca/>

This document does not constitute a record of the structural integrity of the building nor suitability of the design assumptions made. Nordic Structures is responsible only for the structural adequacy of its component based on the design criteria and loadings shown on the calculation sheets.

Document prepared for the use of Stephanie Gon from Alpa, Ontario. (Nordic Request 1810-095)

