

Builder: Gold Park

Project: Pine Valley

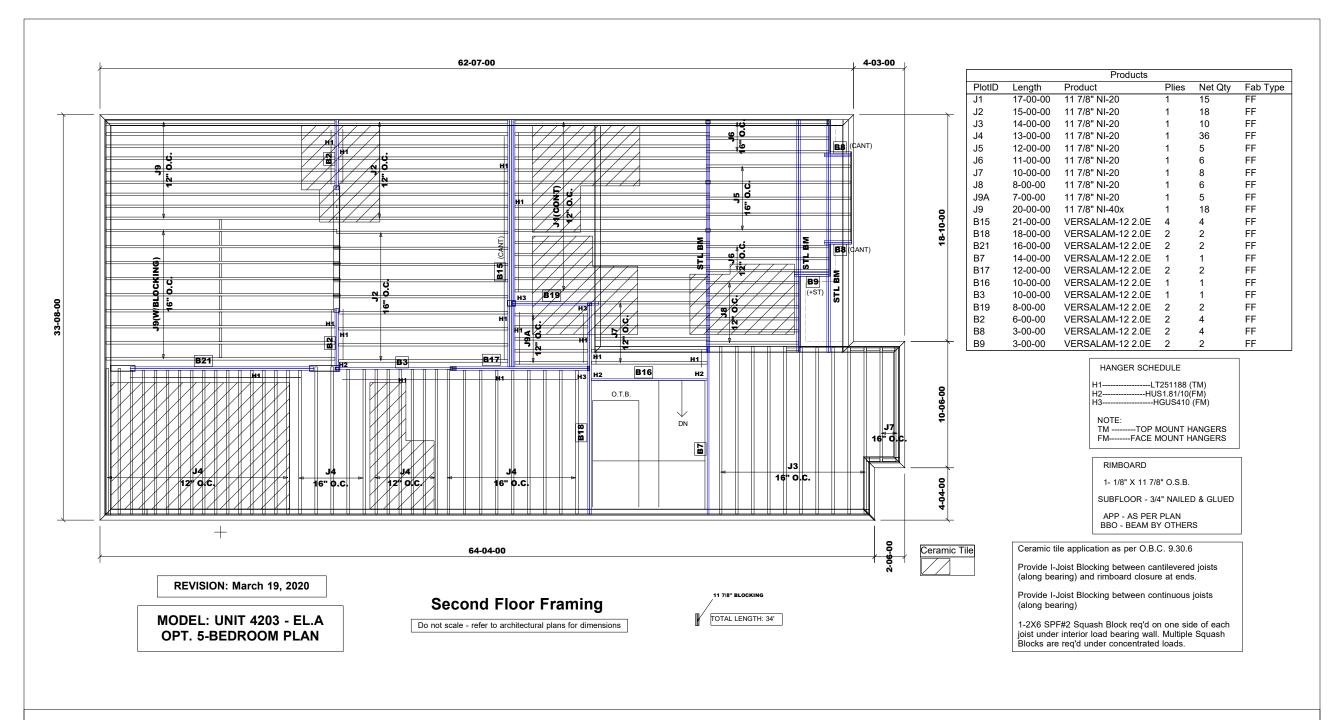
Location: Vaughan

Date: November 12, 2017

Designer: NL/FC

Sheet: 1 of 21

Alpa Roof Trusses Inc. Maple, Ontario Salesperson: Derek



Builder: Gold Park

Project: Pine Valley

Location: Vaughan

Date: November 12, 2017

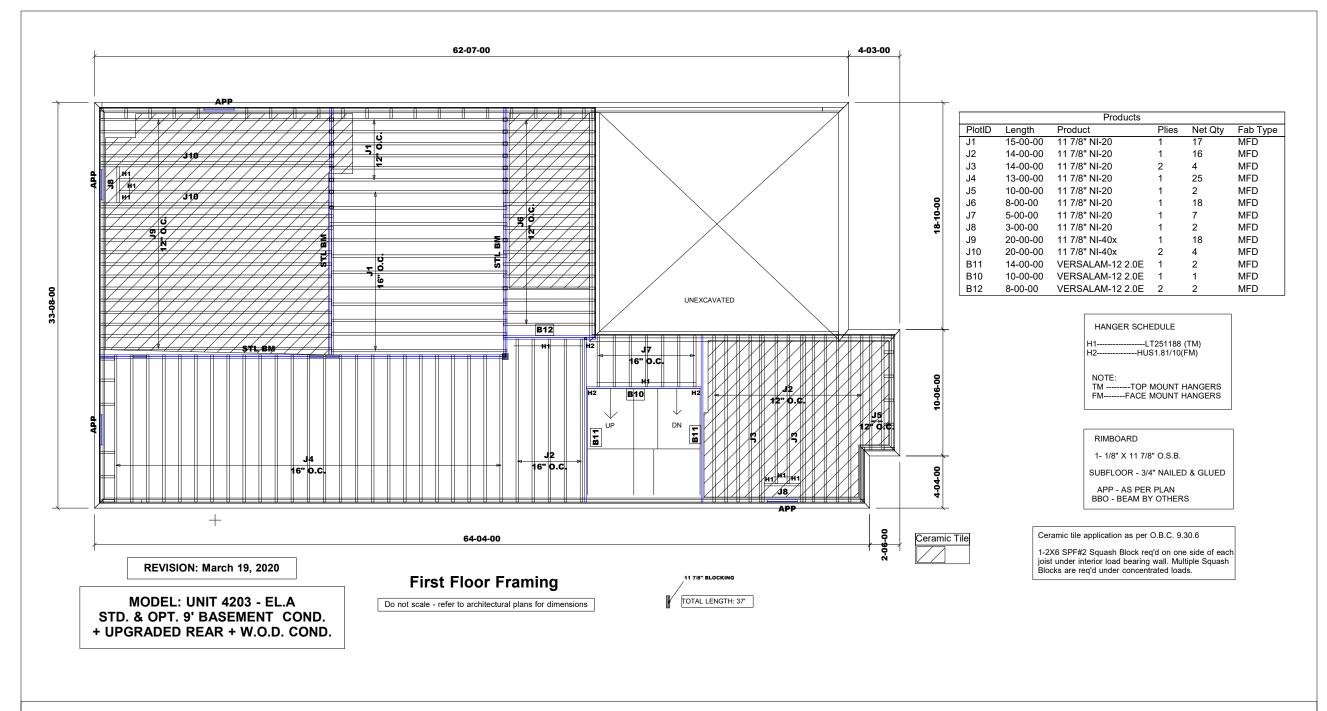
Designer: NL/FC

Sheet: 2 of 21

Alpa Roof Trusses Inc.

Maple, Ontario

Salesperson: Derek



Builder: Gold Park
Project: Pine Valley

Location: Vaughan

Date: November 12, 2017

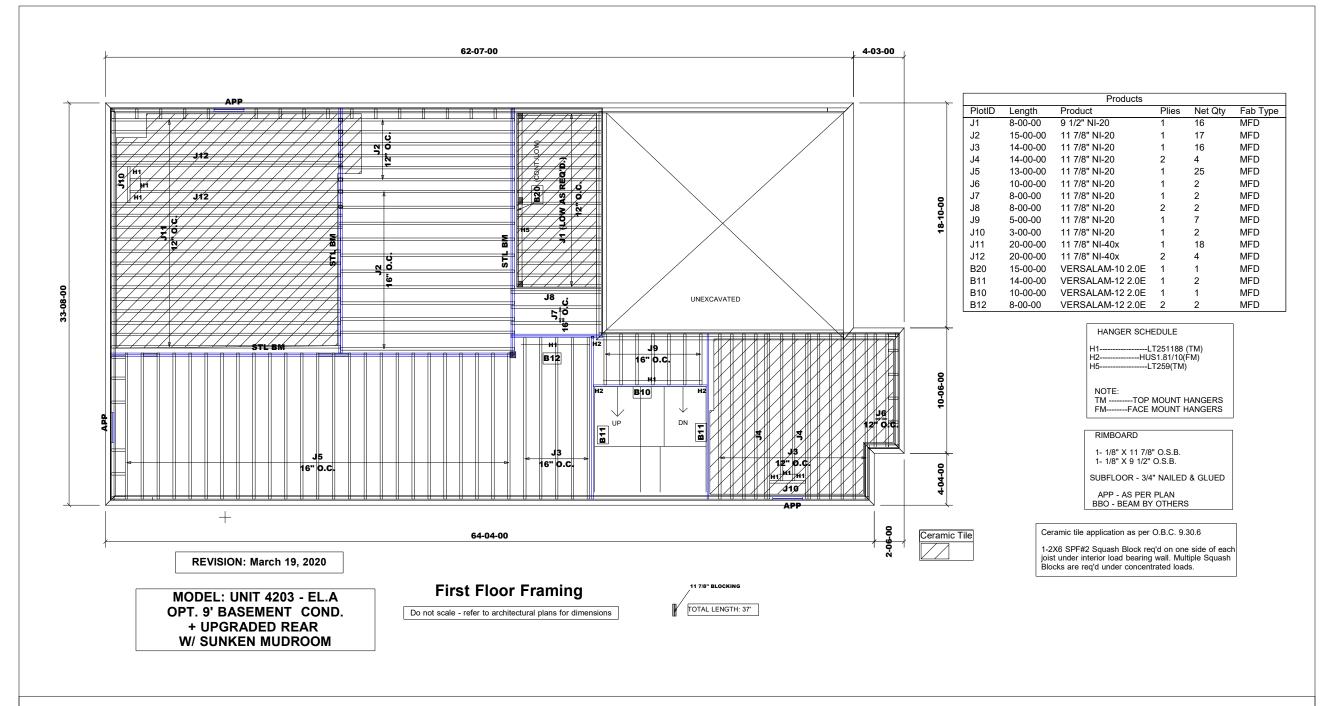
Designer: NL/FC

Sheet: 3 of 21

Alpa Roof Trusses Inc.

Maple, Ontario

Salesperson: Derek



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Project: Pine Valley

Location: Vaughan

Date: November 12, 2017

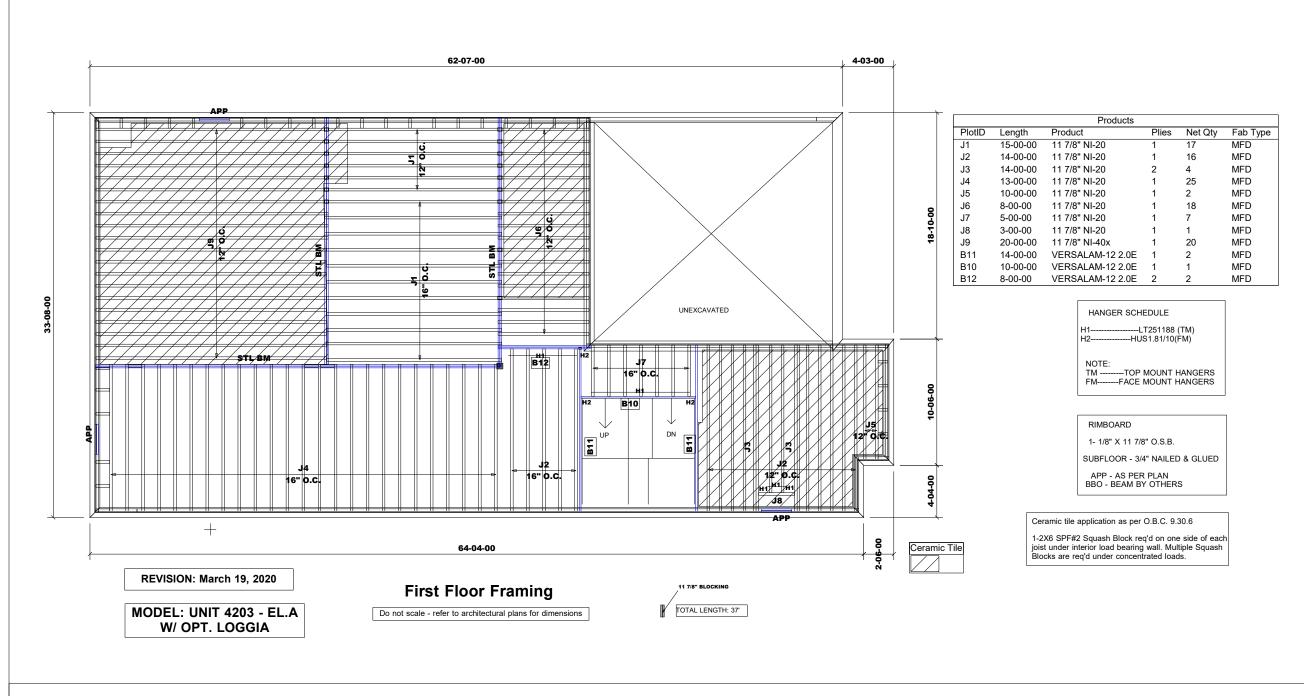
Designer: NL/FC

Sheet: 4 of 21

Alpa Roof Trusses Inc.

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Project: Pine Valley

Location: Vaughan

Date: November 12, 2017

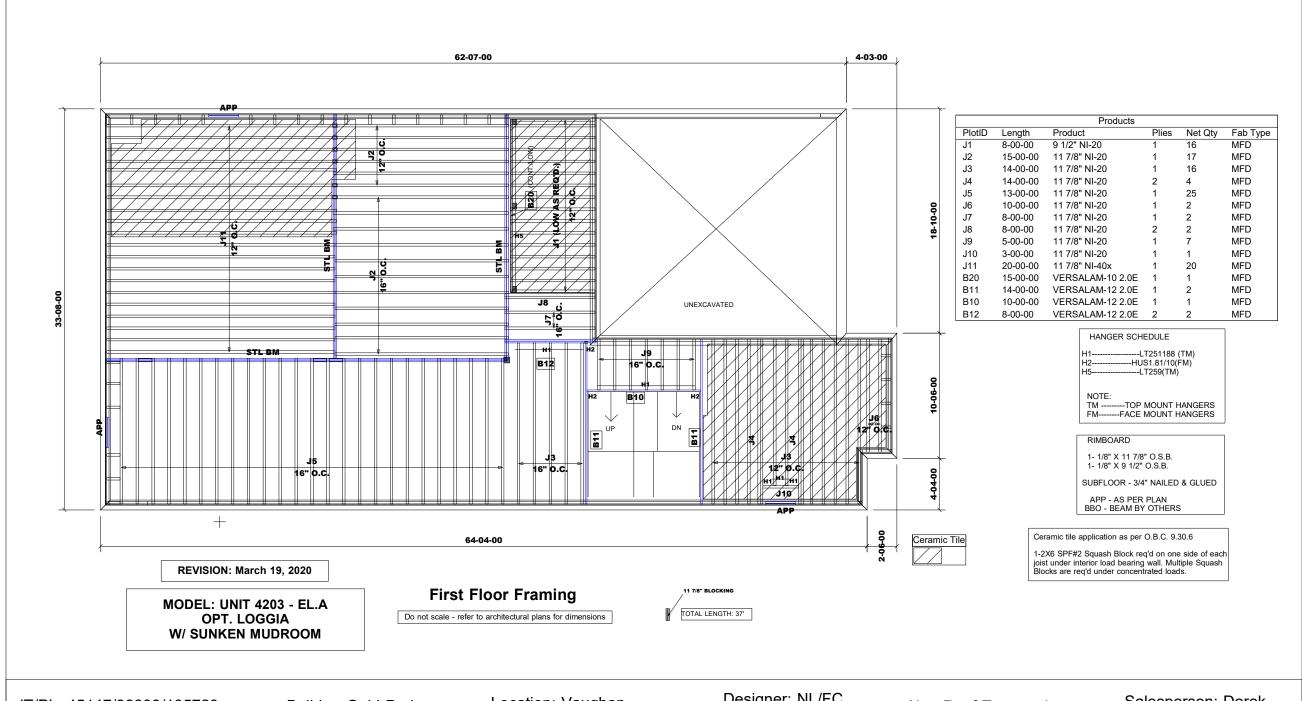
Designer: NL/FC

Sheet: 5 of 21

Alpa Roof Trusses Inc.

Maple, Ontario

Salesperson: Derek



Builder: Gold Park Project: Pine Valley Location: Vaughan

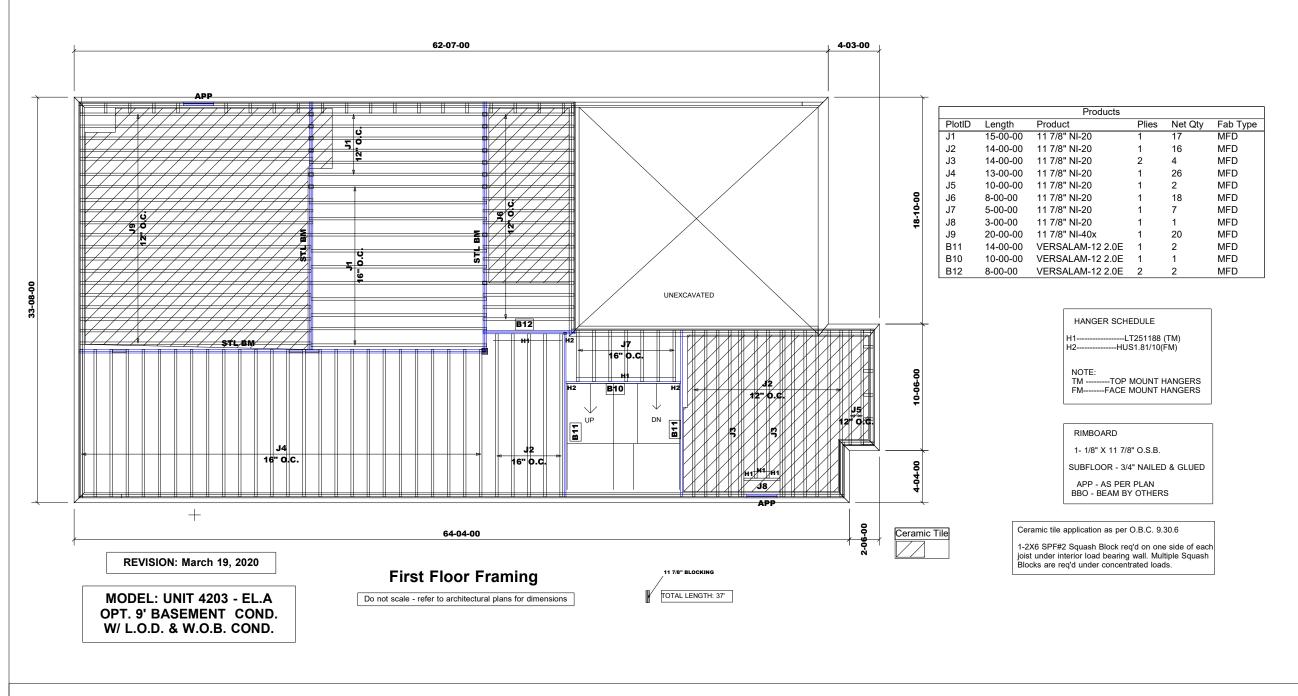
Date: November 12, 2017

Designer: NL/FC

Sheet: 6 of 21

Alpa Roof Trusses Inc. Maple, Ontario

Salesperson: Derek



Builder: Gold Park
Project: Pine Valley

Location: Vaughan

Date: November 12, 2017

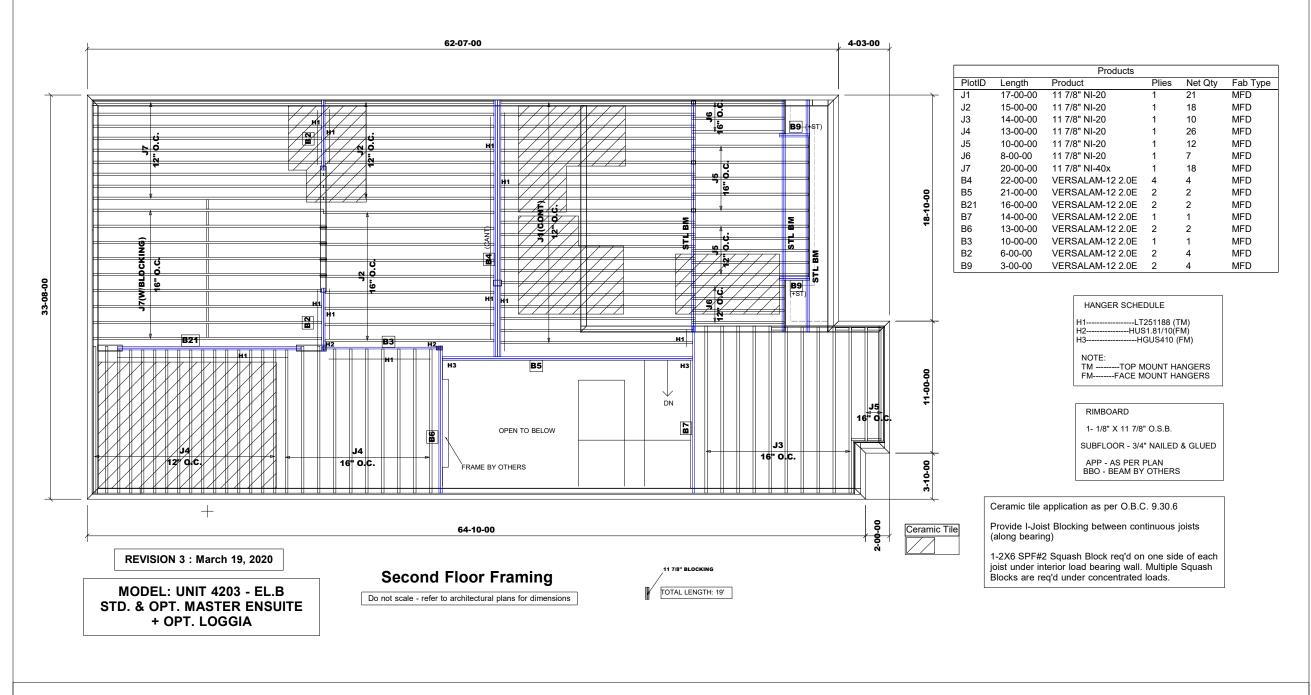
Designer: NL/FC

Sheet: 7 of 21

Alpa Roof Trusses Inc.

Maple, Ontario

Salesperson: Derek



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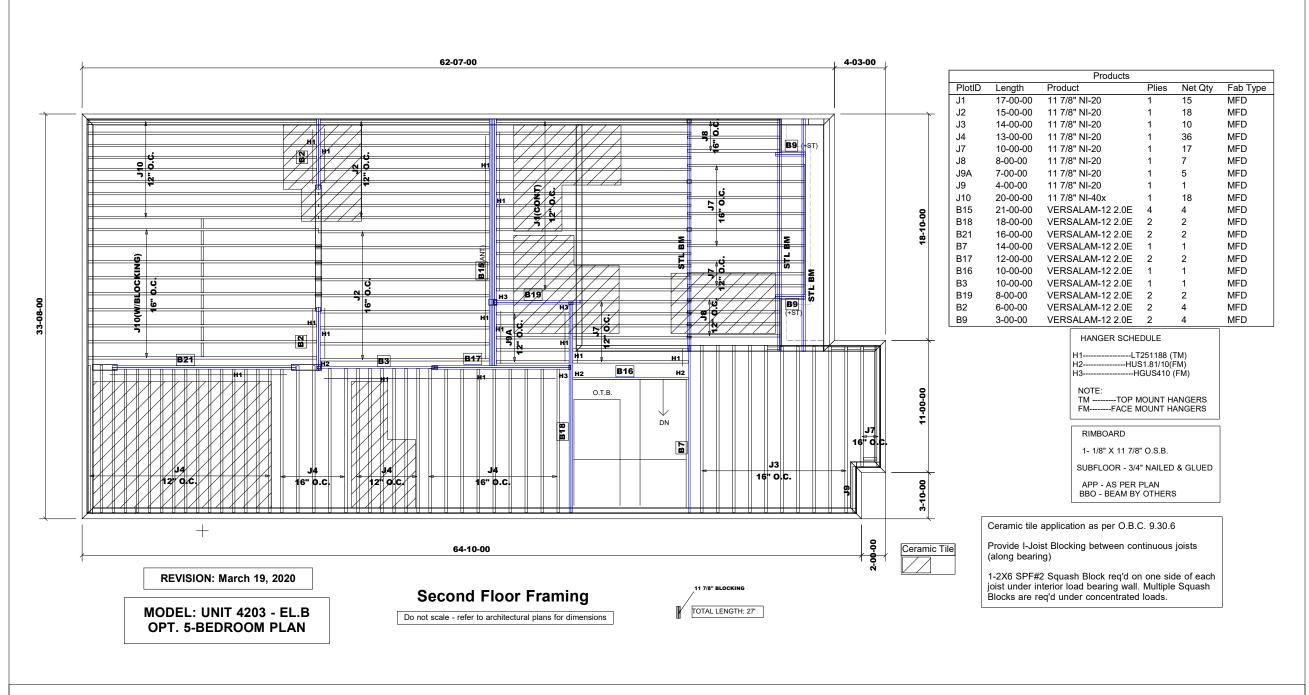
Location: Vaughan

Date: November 12, 2017

Designer: NL/FC

Sheet: 8 of 21

Alpa Roof Trusses Inc. Maple, Ontario Salesperson: Derek



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Project: Pine Valley

Location: Vaughan

Date: November 12, 2017

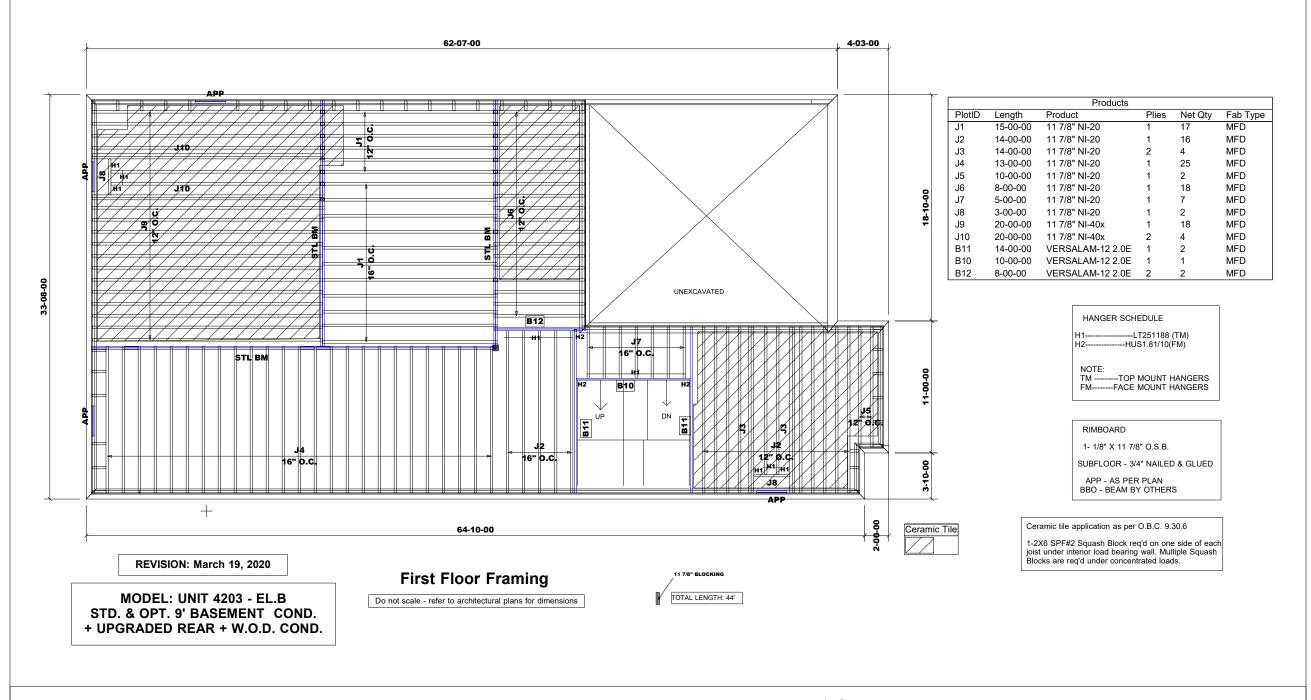
Designer: NL/FC

Sheet: 9 of 21

Alpa Roof Trusses Inc.

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Date: November 12, 2017

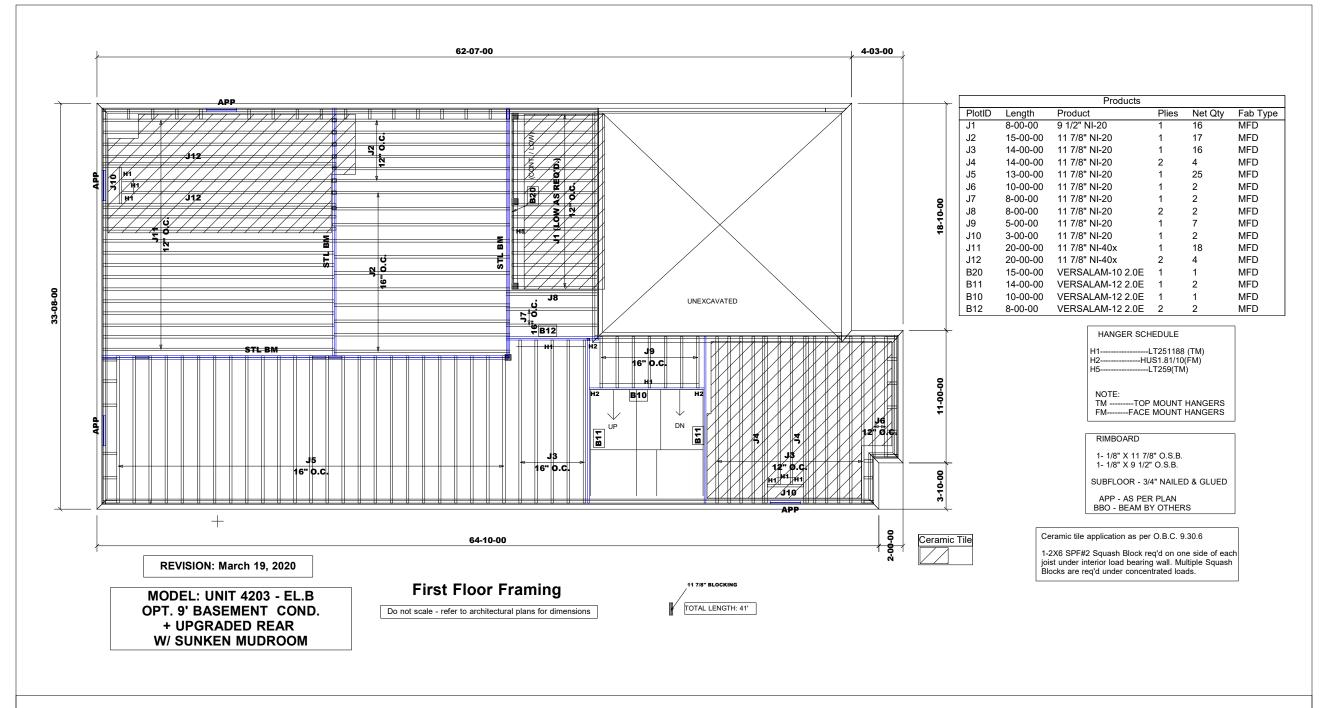
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Sheet: 10 of 21

Alpa Roof Trusses Inc.

Maple, Ontario

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Project: Pine Valley

Location: Vaughan

Date: November 12, 2017

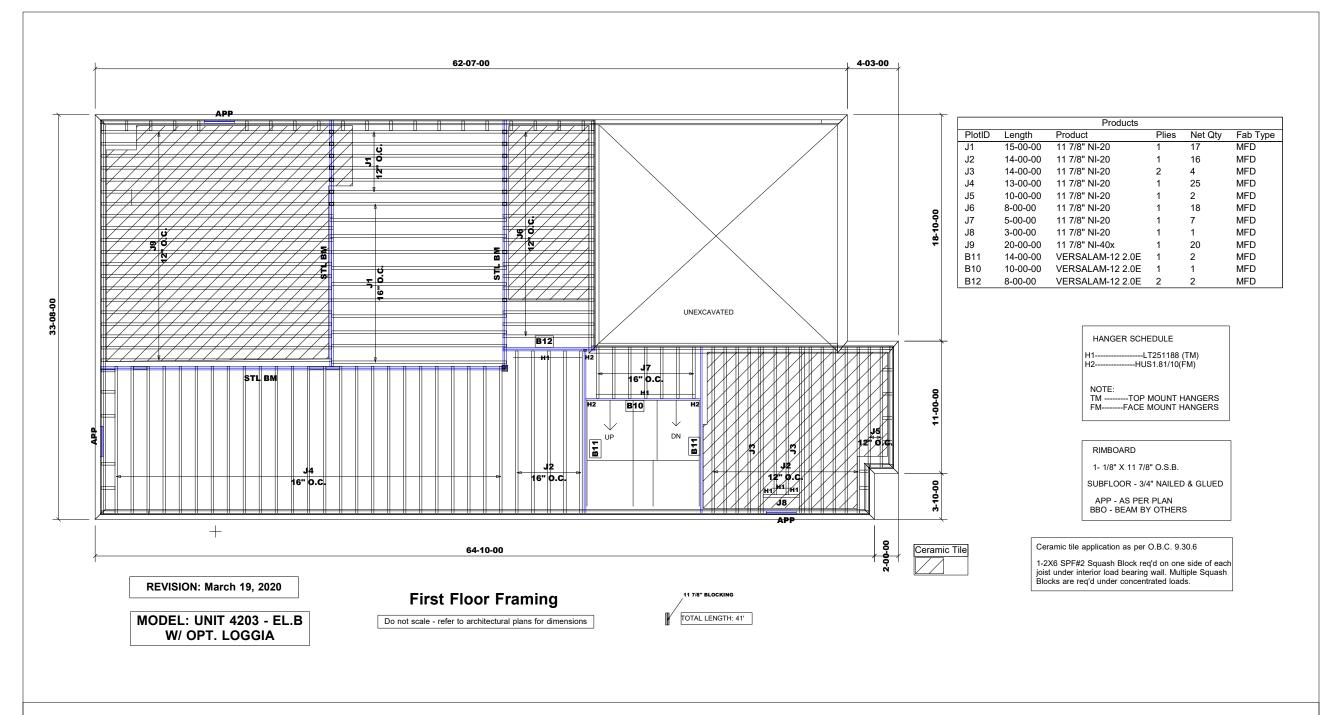
Designer: NL/FC

Sheet: 11 of 21

Alpa Roof Trusses Inc.

Maple, Ontario

Salesperson: Derek



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Project: Pine Valley

Location: Vaughan

Date: November 12, 2017

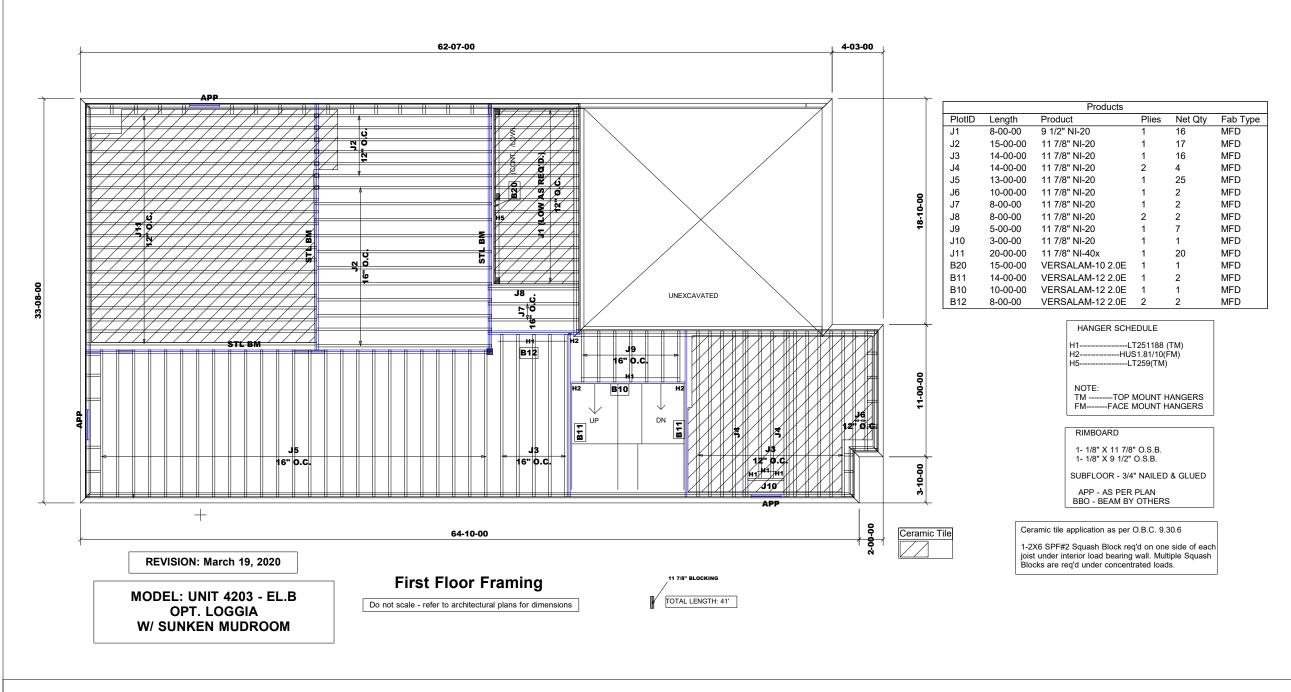
Designer: NL/FC

Sheet: 12 of 21

Alpa Roof Trusses Inc.

Maple, Ontario

Salesperson: Derek



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Project: Pine Valley

Location: Vaughan

Date: November 12, 2017

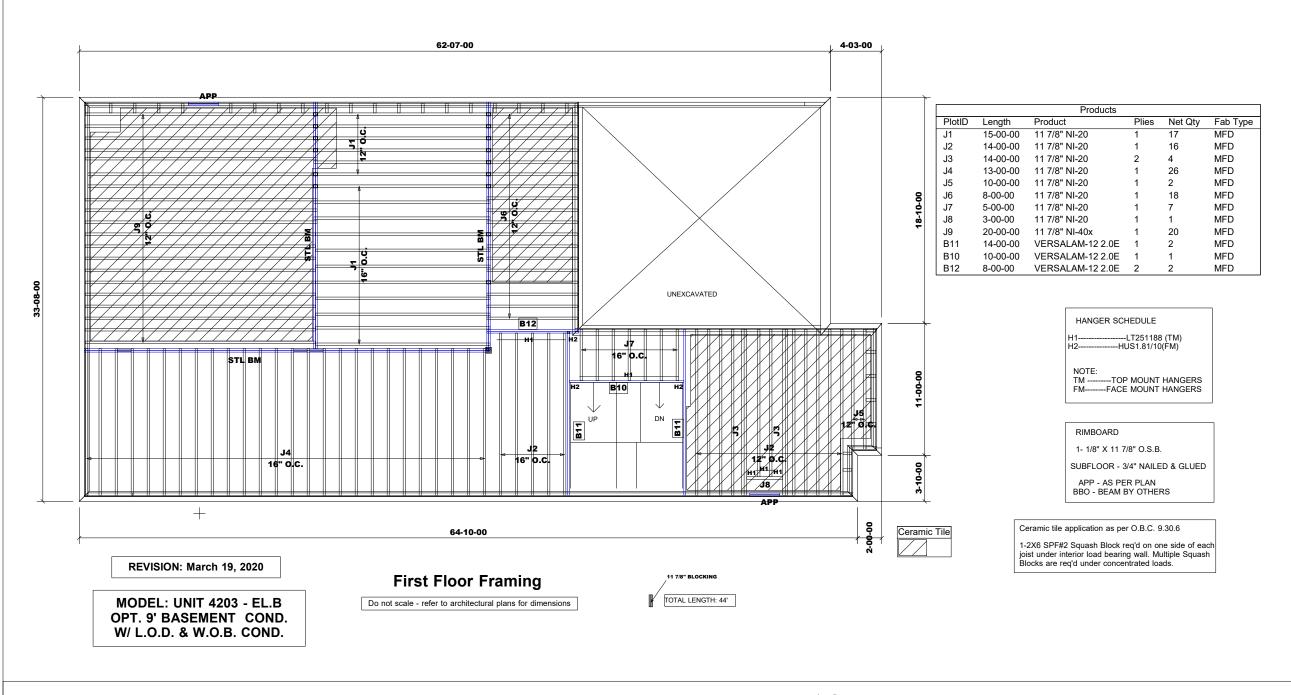
Designer: NL/FC

Sheet: 13 of 21

Alpa Roof Trusses Inc.

Maple, Ontario

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Location: Vaughan

Date: November 12, 2017

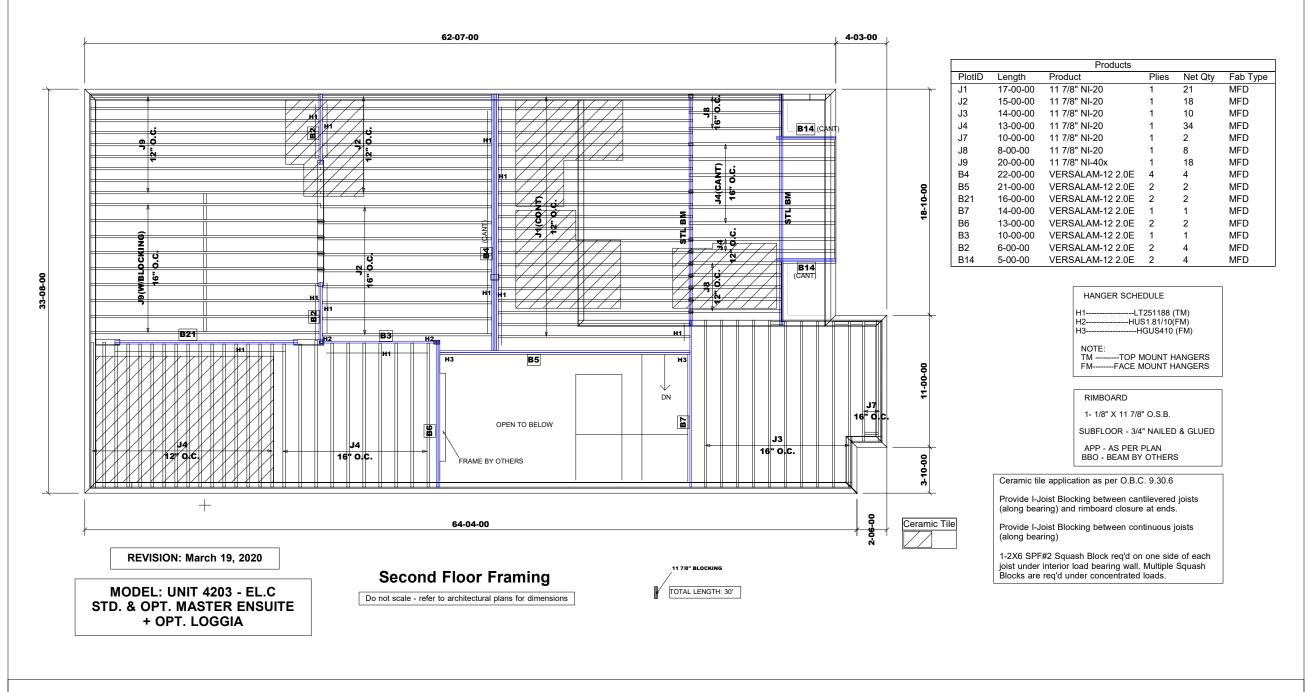
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Sheet: 14 of 21

Alpa Roof Trusses Inc.

Maple, Ontario

Salesperson: Derek



Builder: Gold Park
Project: Pine Valley

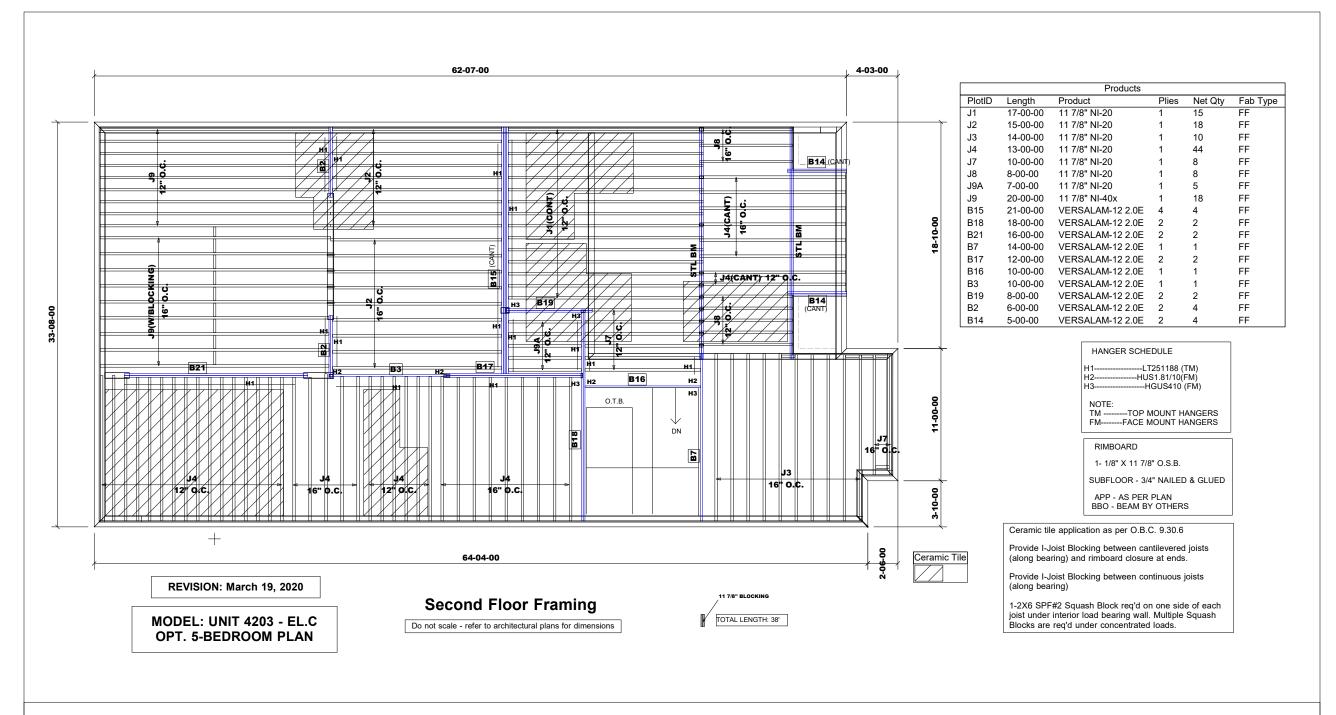
Location: Vaughan

Date: November 12, 2017

Designer: NL/FC

Sheet: 15 of 21

Alpa Roof Trusses Inc. Maple, Ontario Salesperson: Derek



Builder: Gold Park

Project: Pine Valley

Location: Vaughan

Date: November 12, 2017

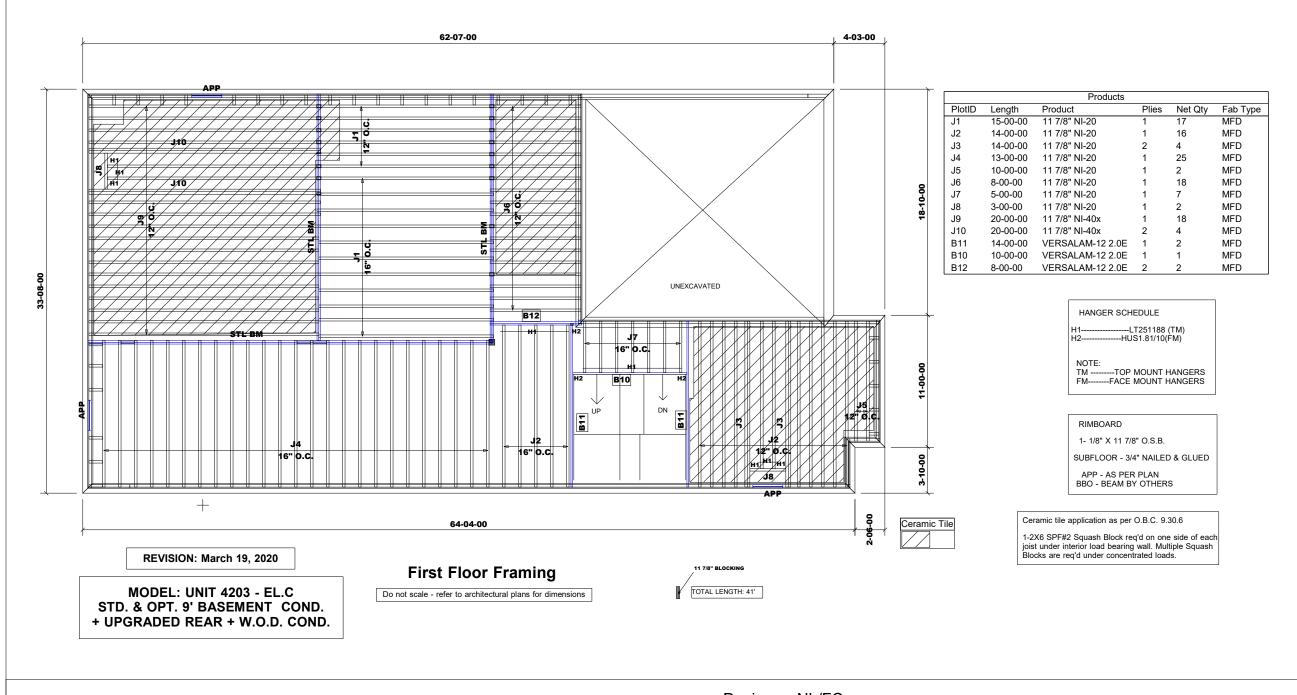
Designer: NL/FC

Sheet: 16 of 21

Alpa Roof Trusses Inc.

Maple, Ontario

Salesperson: Derek



Builder: Gold Park
Project: Pine Valley

Location: Vaughan

Date: November 12, 2017

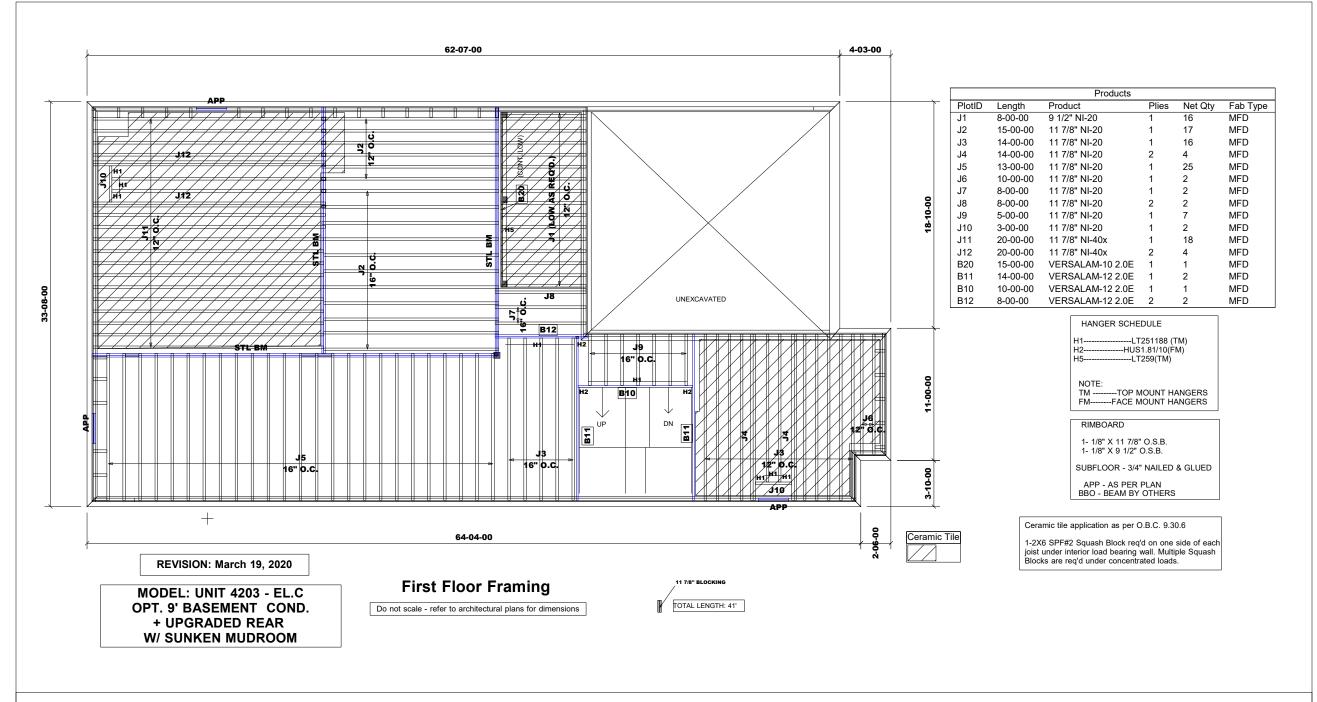
Designer: NL/FC

Sheet: 17 of 21

Alpa Roof Trusses Inc.

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Project: Pine Valley

Location: Vaughan

Date: November 12, 2017

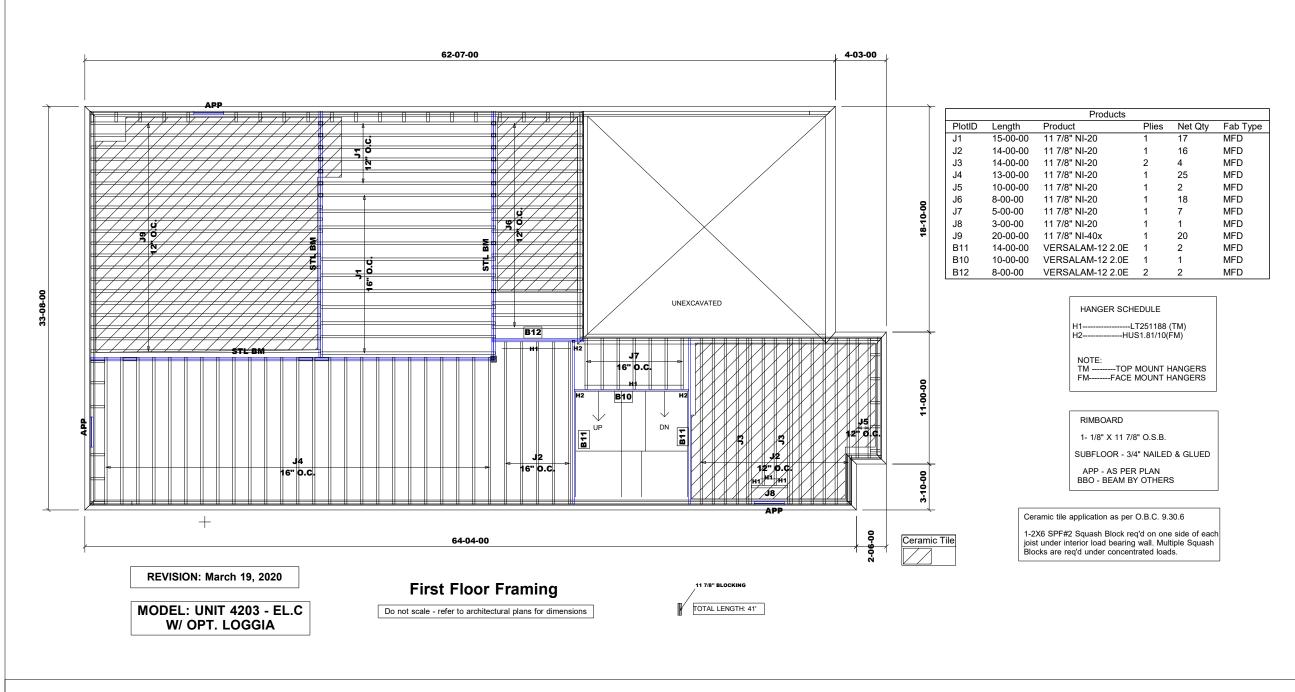
Designer: NL/FC

Sheet: 18 of 21

Alpa Roof Trusses Inc.

Maple, Ontario

Salesperson: Derek



Builder: Gold Park
Project: Pine Valley

Location: Vaughan

Date: November 12, 2017

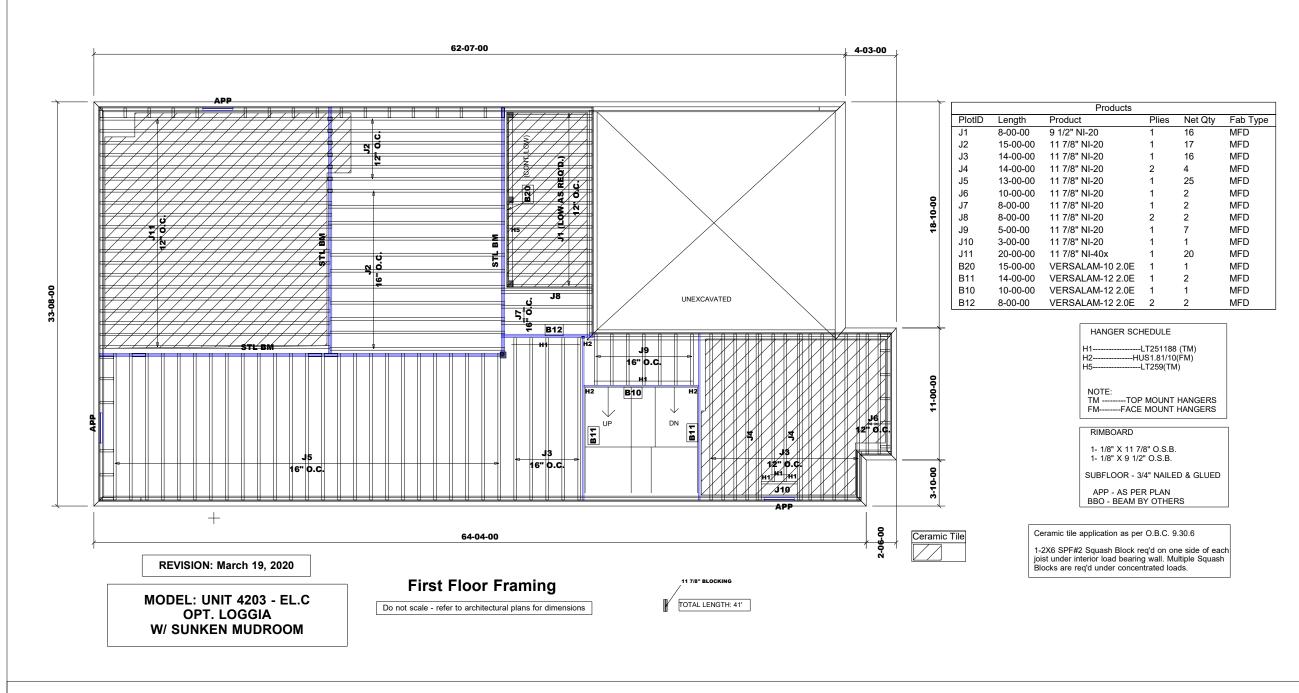
Designer: NL/FC

Sheet: 19 of 21

Alpa Roof Trusses Inc.

Maple, Ontario

Salesperson: Derek



Builder: Gold Park
Project: Pine Valley

Date: November 12, 2017

Location: Vaughan

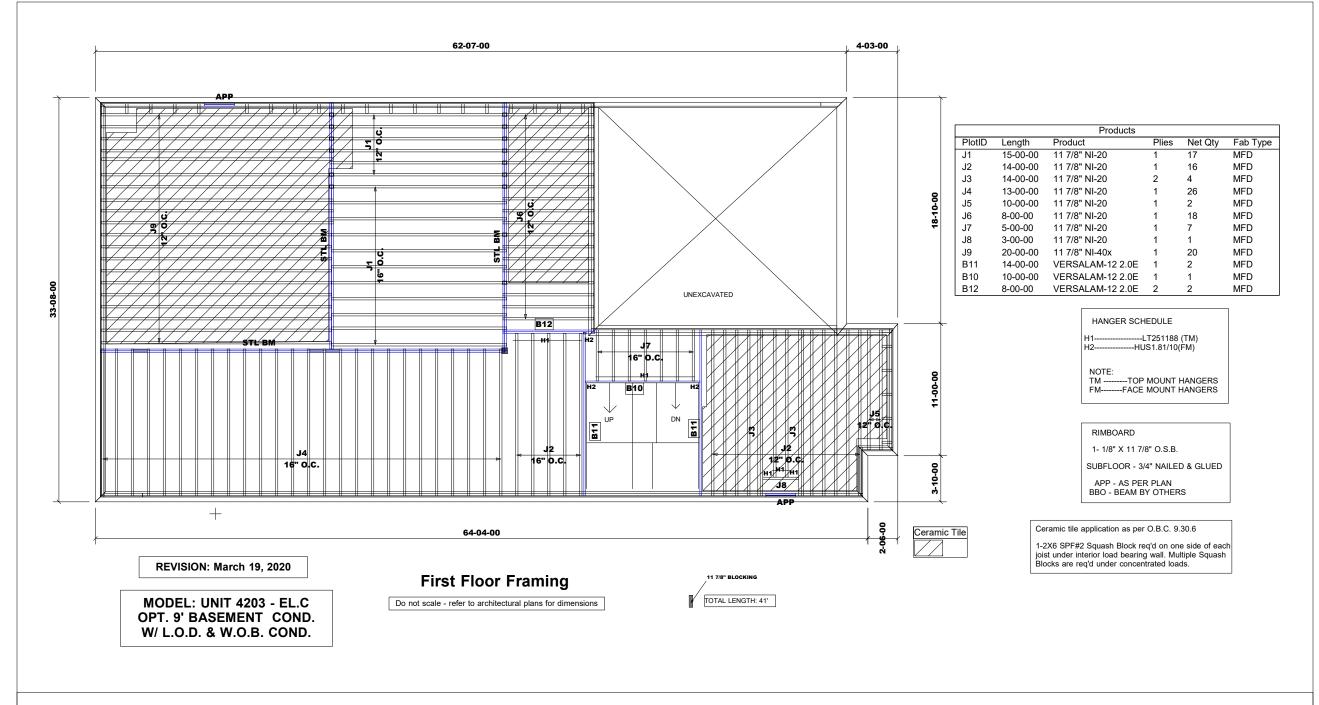
Designer: NL/FC

Sheet: 20 of 21

Alpa Roof Trusses Inc.

Maple, Ontario

Salesperson: Derek



Builder: Gold Park
Project: Pine Valley

Location: Vaughan

Date: November 12, 2017

Designer: NL/FC

Sheet: 21 of 21

Alpa Roof Trusses Inc.

Maple, Ontario

Salesperson: Derek





# Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

# **B02** (Floor Beam)

File name:

Specifier:

290672

NL

**PASSED** 

March 19, 2020 14:32:52

**BC CALC® Member Report** 

Dry | 1 span | No cant.

**Build 7555** 

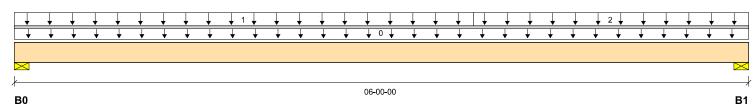
45147 (4203) Job name:

Description: Pine Valley Address: Second Floor Framing

City, Province, Postal Code: Vaughan, ON

Builder: Gold Park Designer:

Code reports: CCMC 12472-R Company: Alpa Roof Trusses



### **Total Horizontal Product Length = 06-00-00**

Reaction Summary (Down / Uplift) (lbs)

Live Dead Snow B0, 3-1/2" 2020 / 0 1046 / 0 B1, 3-1/2" 1940 / 0 1006 / 0

Lo	Load Summary							Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	06-00-00	Тор		12			00-00-00
1		Unf. Area (Ib/ft²)	L	00-00-00	03-09-00	Top	40	20			17-00-00
2		Unf. Area (Ib/ft²)	L	03-09-00	06-00-00	Тор	40	20			15-08-00

		Factored	Demand/		
Controls Summary	Factored Demand	Resistance	Resistance	Case	Location
Pos. Moment	5490 ft- <b>I</b> bs	35392 ft <b>-l</b> bs	15.5%	1	02-11-12
End Shear	2468 lbs	14464 <b>I</b> bs	17.1%	1	01-03-06
Total Load Deflection	L/999 (0.022")	n\a	n\a	4	02-11-12
Live Load Deflection	L/999 (0.014")	n\a	n\a	5	02-11-12
Max Defl.	0.022"	n\a	n\a	4	02-11-12
Span / Depth	5.6				

Bearing	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
В0	Wall/Plate	3-1/2" x 3-1/2"	4338 <b>I</b> bs	57.6%	29.0%	Spruce-Pine-Fir
B1	Wall/Plate	3-1/2" x 3-1/2"	4167 lbs	55.3%	27.9%	Spruce-Pine-Fir

### **Notes**

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4

NAIL ONE PLY TO ANOTHER WITH 3-1/2" SPIRAL NAILS @ O/C, STAGGERED IN 2 ROWS





**BC CALC® Member Report** 



# B03 (Floor Beam)

Specifier:

Dry | 1 span | No cant.

March 19, 2020 14:32:52

**PASSED** 

**Build 7555** 

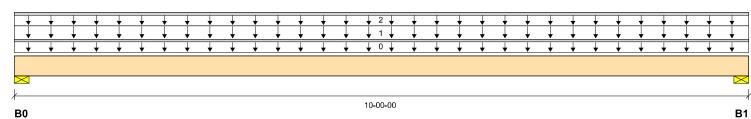
45147 (4203) Job name:

290672 File name: Pine Valley Address: Description: Second Floor Framing

City, Province, Postal Code: Vaughan, ON

Builder: Gold Park Designer: NL

Alpa Roof Trusses CCMC 12472-R Company: Code reports:



### Total Horizontal Product Length = 10-00-00

Reaction Summary (Down / Uplift) (Ibs)

Bearing	Live	` Dead	Snow	Wind
B0, 3-1/2"	1400 / 0	855 / 0		
B1, 3-1/2"	1400 / 0	855 / 0		

	ad Summary	Land Tons	D-f	C4 4	F., .1	1	Live	Dead	Snow	Wind	Tributary
rag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	10-00-00	Тор		6			00-00-00
1		Unf. Area (lb/ft²)	L	00-00-00	10-00-00	Top	40	15			07-00-00

10-00-00

Top

00-00-00

		Factored	Demand/		
Controls Summary	Factored Demand	Resistance	Resistance	Case	Location
Pos. Moment	7213 ft- <b>I</b> bs	17696 ft- <b>I</b> bs	40.8%	1	05-00-00
End Shear	2357 lbs	7232 lbs	32.6%	1	01-03-06
Total Load Deflection	L/665 (0.172")	n\a	36.1%	4	05-00-00
Live Load Deflection	L/999 (0.107")	n\a	n\a	5	05-00-00
Max Defl.	0.172"	n\a	17.2%	4	05-00-00
Span / Depth	9.6				

1

Unf. Lin. (lb/ft)

_Bearin	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	3-1/2" x 1-3/4"	3169 <b>l</b> bs	84.1%	42.4%	Spruce-Pine-Fir
B1	Wall/Plate	3-1/2" x 1-3/4"	3169 <b>l</b> bs	84.1%	42.4%	Spruce-Pine-Fir

### **Notes**

2

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4



n∖a

# **Disclosure**

60

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

BC CALC®, BC FRAMER®, AJS™ ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,

SE007754





B04 (Floor Beam)

File name:

Specifier:

290672

PASSED

March 19, 2020 14:32:52

**BC CALC® Member Report** 

Dry | 2 spans | L cant.

**Build 7555** 

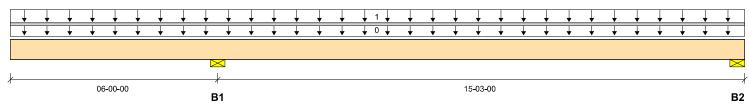
45147 (4203) Job name:

Pine Valley Address: Description: Second Floor Framing

City, Province, Postal Code: Vaughan, ON Builder:

Gold Park Designer: NL

Code reports: CCMC 12472-R Company: Alpa Roof Trusses



Total Horizontal Product Length = 21-03-00

Reaction Summary (Down / Uplift) (lbs)

Live Snow B1, 3-1/2" 6472 / 0 3590 / 0 B2, 3-1/2" 3405 / 527 1597 / 0

Loa	Load Summary							Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	21-03-00	Тор		24			00-00-00
1		Unf. Area (Ib/ft²)	L	00-00-00	21-03-00	Top	40	20			11-00-00

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	24541 ft- <b>l</b> bs	73615 ft- <b>l</b> bs	33.3%	3	13-10-04
Neg. Moment	-17372 ft- <b>l</b> bs	-73615 ft-lbs	23.6%	1	06-00-00
End Shear	5867 lbs	28927 lbs	20.3%	3	19-11-10
Cont. Shear	7309 lbs	28927 lbs	25.3%	1	07-01-10
Total Load Deflection	L/520 (0.346")	n\a	46.1%	10	13-09-00
Live Load Deflection	2xL/437 (-0.33")	n\a	82.4%	13	00-00-00
Total Neg. Defl.	2xL/399 (-0.361")	n\a	60.2%	10	00-00-00
Max Defl.	0.346"	n\a	34.6%	10	13-09-00
Cant. Max Defl.	-0.361"	n\a	36.1%	10	00-00-00
Span / Depth	15.2				

Bearing	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Wall/Plate	3-1/2" x 7"	14195 lbs	94.2%	47.5%	Spruce-Pine-Fir
B2	Wall/Plate	3-1/2" x 7"	7104 <b>I</b> bs	47.1%	23.8%	Spruce-Pine-Fir

### Cautions

Long Cantilever: Sheathing required on bottom flange and adjacent back span or bracing designed by the design professional of record. Design professional of record must address uplift at supports.

### **Notes**

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets User specified (2xL/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Design meets arbitrary (1") Cantilever Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4

Cantilevers require sheathed bottom flanges, blocking at cantilever support and closure at ends.



NAIL ONE PLY TO ANOTHER WITH 3-1/2" SPIRAL NAILS @ STAGGERED IN 2 ROWS

PLUS, SDW22634 SIMPSON WOOD SCREW @ 12" O/C, STAGGERED IN 2 ROWS.





# Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

# **B05** (Floor Beam)

File name:

Specifier:

290672

PASSED

**BC CALC® Member Report** 

Dry | 1 span | No cant.

March 19, 2020 14:32:52

**Build 7555** 

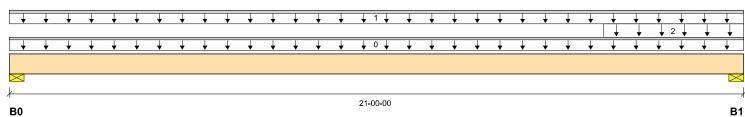
Job name: 45147 (4203)

Address: Pine Valley Description: Second Floor Framing

City, Province, Postal Code: Vaughan, ON Builder: Gold Park

Gold Park Designer: NL

Code reports: CCMC 12472-R Company: Alpa Roof Trusses



### Total Horizontal Product Length = 21-00-00

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind	
B0, 3-1/2"	339 / 0	294 / 0			
B1, 3-1/2"	868 / 0	493 / 0			

Loa	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	21-00-00	Тор		12			00-00-00
1		Unf. Lin. (lb/ft)	L	00-00-00	21-00-00	Тор	27	14			n∖a
2		Unf. Area (lb/ft²)	L	17-00-00	21-00-00	Тор	40	15			04-00-00

0 1 - 0		Factored	Demand/		
Controls Summary	Factored Demand	Resistance	Resistance	Case	Location
Pos. Moment	5053 ft-lbs	35392 ft <b>-l</b> bs	14.3%	1	12-01-01
End Shear	1421 <b>l</b> bs	14464 <b>I</b> bs	9.8%	1	19-08-10
Total Load Deflection	L/852 (0.289")	n\a	28.2%	4	10-10-05
Live Load Deflection	L/1513 (0.163")	n\a	23.8%	5	10-10-05
Max Defl.	0.289"	n\a	28.9%	4	10-10-05
Span / Depth	20.8				

Bearing	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	3-1/2" x 3-1/2"	876 lbs	11.6%	5.9%	Spruce-Pine-Fir
B1	Wall/Plate	3-1/2" x 3-1/2"	1918 <b>I</b> bs	25.4%	12.8%	Spruce-Pine-Fir



### **Notes**

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4

NAIL ONE PLY TO ANOTHER WITH 3-1/2" SPIRAL NAILS @ 12" O/C,

STAGGERED IN 2 ROWS



# Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

# **B06 (Floor Beam)**

File name:

Specifier:

290672



**PASSED** 

March 19, 2020 14:32:52

BC CALC® Member Report

Dry | 1 span | No cant.

**Build 7555** 

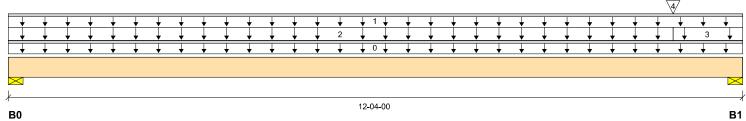
Job name: 45147 (4203)

Address: Pine Valley Description: Second Floor Framing

City, Province, Postal Code: Vaughan, ON

Builder: Gold Park Designer: NL

Code reports: CCMC 12472-R Company: Alpa Roof Trusses



### Total Horizontal Product Length = 12-04-00

Reaction Summary (Down / Uplift) (lbs)

 Bearing
 Live
 Dead
 Snow
 Wind

 B0, 3-1/2"
 454 / 0
 652 / 0

 B1, 3-1/2"
 1162 / 0
 1058 / 0

Lo	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	12-04-00	Тор		12			00-00-00
1		Unf. Lin. (Ib/ft)	L	00-00-00	12-04-00	Тор	27	74			n\a
2		Unf. Area (Ib/ft²)	L	00-00-00	11-02-00	Тор	40	15			01-00-00
3		Unf. Area (Ib/ft²)	L	11-02-00	12-04-00	Top	40	15			10-08-00
4		Conc. Pt. (lbs)	L	11-02-00	11-02-00	Тор	339	294			n∖a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	4596 ft- <b>l</b> bs	35392 ft <b>-l</b> bs	13.0%	1	06-07-08
End Shear	2529 lbs	14464 <b>I</b> bs	17.5%	1	11-00-10
Total Load Deflection	L/999 (0.09")	n\a	n\a	4	06-03-13
Live Load Deflection	L/999 (0.038")	n\a	n\a	5	06-03-13
Max Defl.	0.09"	n\a	n\a	4	06-03-13
Span / Depth	12.0				

Bearing	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	3-1/2" x 3-1/2"	1496 lbs	19.8%	10.0%	Spruce-Pine-Fir
B1	Wall/Plate	3-1/2" x 3-1/2"	3065 lbs	40.7%	20.5%	Spruce-Pine-Fir



### **Notes**

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4

NAIL ONE PLY TO ANOTHER WITH 3-1/2" SPIRAL NAILS @ 12" O/C, STAGGERED IN 2 ROWS



# Single 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

# **B07** (Floor Beam)

File name:

Specifier:

290672

**PASSED** 

**BC CALC® Member Report** 

Dry | 1 span | No cant.

March 19, 2020 14:32:52

**Build 7555** 

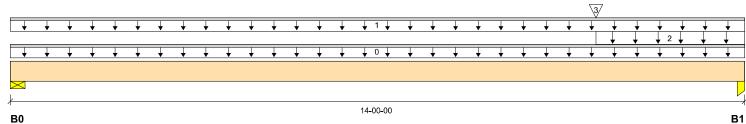
45147 (4203) Job name:

Pine Valley Address: Description: Second Floor Framing

City, Province, Postal Code: Vaughan, ON Builder:

Gold Park Designer: NL

CCMC 12472-R Company: Alpa Roof Trusses Code reports:



## Total Horizontal Product Length = 14-00-00

# Reaction Summary (Down / Uplift) (lbs)

**Bearing** Live Dead Snow Wind B0, 3-1/2" 438 / 0 686 / 0 B1, 3-1/2" 1742 / 0 1278 / 0

Lo	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag	•	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	14-00-00	Тор		6			00-00-00
1		Unf. Lin. (lb/ft)	L	00-00-00	14-00-00	Top	27	74			n\a
2		Unf. Area (lb/ft²)	L	11-02-00	14-00-00	Top	40	15			08-03-00
3		Conc. Pt. (lbs)	L	11-02-00	11-02-00	Top	868	493			n∖a

		Factored	Demand/		
Controls Summary	Factored Demand	Resistance	Resistance	Case	Location
Pos. Moment	7823 ft- <b>I</b> bs	17696 ft <b>-I</b> bs	44.2%	1	10-10-04
End Shear	3198 lbs	7232 <b>l</b> bs	44.2%	1	12-08-10
Total Load Deflection	L/445 (0.365")	n\a	54.0%	4	07-05-08
Live Load Deflection	L/927 (0.175")	n\a	38.8%	5	07-09-03
Max Defl.	0.365"	n\a	36.5%	4	07-05-08
Span / Depth	13.7				

Bearing	յ Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	3-1/2" x 1-3/4"	1515 lbs	40.2%	20.3%	Spruce-Pine-Fir
B1	Column	3-1/2" x 1-3/4"	4211 <b>I</b> bs	39.6%	56.4%	Spruce-Pine-Fir

### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4



# **Disclosure**

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SE007758





# Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

# 3" VERSA-LAM® 2.0 3100 SI

290672

NL



March 19, 2020 14:32:52

**BC CALC® Member Report** 

B08 (Floor Beam)

File name:

Specifier:

Dry | 2 spans | R cant.

**Build 7555** 

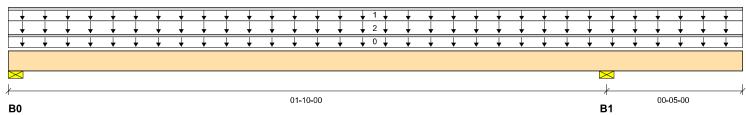
Job name: 45147 (4203)

Address: Pine Valley Description: Second Floor Framing

City, Province, Postal Code: Vaughan, ON

Builder: Gold Park Designer:

Code reports: CCMC 12472-R Company: Alpa Roof Trusses



### Total Horizontal Product Length = 02-03-00

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B0, 3-1/2"	28 / 1	202 / 0	127 / 0	
B1. 3-1/2"	34 / 0	263 / 0	160 / 0	

Loa	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	02-03-00	Тор		12			00-00-00
1		Unf. Lin. (lb/ft)	L	00-00-00	02-03-00	Top	27	114			n∖a
2		Unf. Area (lb/ft²)	L	00-00-00	02-03-00	Top		13	21		06-00-00

		Factored	Demand/		
Controls Summary	Factored Demand	Resistance	Resistance	Case	Location
Pos. Moment	138 ft <b>-l</b> bs	35392 ft <b>-l</b> bs	0.4%	26	00-11-14
Neg. Moment	-41 ft- <b>l</b> bs	-35392 ft- <b>l</b> bs	0.1%	28	01-10-00
End Shear	144 lbs	14464 lbs	1.0%	31	01-03-06
Cont. Shear	-139 lbs	-14464 <b>I</b> bs	1.0%	26	00-08-06
Total Load Deflection	L/999 (0")	n\a	n\a	54	01-00-02
Max Defl.	0"	n\a	n∖a	54	01-00-02
Span / Depth	1.6				

Beari	ing Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	3-1/2" x 3-1/2"	470 lbs	6.2%	3.1%	Spruce-Pine-Fir
B1	Wall/Plate	3-1/2" x 3-1/2"	603 lbs	8.0%	4.0%	Spruce-Pine-Fir



### **Notes**

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4

Cantilevers require sheathed bottom flanges, blocking at cantilever support and closure at ends.

NAIL ONE PLY TO ANOTHER WITH 3-1/2" SPIRAL NAILS @ 3" O/C, STAGGERED IN 2 ROWS





# Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

# B09 (Floor Beam)

File name:

Specifier:

290672

PASSED

**BC CALC® Member Report** 

Dry | 1 span | No cant.

March 19, 2020 14:32:52

**Build 7555** 

Job name: 45147 (4203)

Address: Pine Valley Description: Second Floor Framing

City, Province, Postal Code: Vaughan, ON Builder: Gold Park

Gold Park Designer: NL

Code reports: CCMC 12472-R Company: Alpa Roof Trusses

B0 03-00-00

B1

### Total Horizontal Product Length = 03-00-00

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B0, 3-1/2"	40 / 0	370 / 0	284 / 0	
B1. 3-1/2"	40 / 0	370 / 0	283 / 0	

Loa	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	03-00-00	Тор		12			00-00-00
1		Unf. Lin. (lb/ft)	L	00-00-00	03-00-00	Тор	27	114			n∖a
2		Unf. Area (lb/ft²)	L	00-00-00	03-00-00	Top		13	21		09-00-00

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
				Case	
Pos. Moment	500 ft <b>-l</b> bs	35392 ft <b>-I</b> bs	1.4%	5	01-06-00
End Shear	135 lbs	14464 <b>I</b> bs	0.9%	5	01-03-06
Total Load Deflection	L/999 (0")	n\a	n\a	11	01-06-00
Live Load Deflection	L/999 (0")	n\a	n\a	15	01-06-00
Max Defl.	0"	n\a	n\a	11	01-06-00
Span / Depth	2.6				

Bearing	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	3-1/2" x 3-1/2"	928 lbs	12.3%	6.2%	Spruce-Pine-Fir
B1	Wall/Plate	3-1/2" x 3-1/2"	928 lbs	12.3%	6.2%	Spruce-Pine-Fir



### **Notes**

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4

NAIL ONE PLY TO ANOTHER WITH 3-1/2" SPIRAL NAILS @ 4" O/C, STAGGERED IN 2 ROWS



# Single 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

# PASSED

### B10 (Floor Beam)

File name:

Specifier:

290672

**BC CALC® Member Report** 

Dry | 1 span | No cant.

March 19, 2020 14:32:52

Build 7555

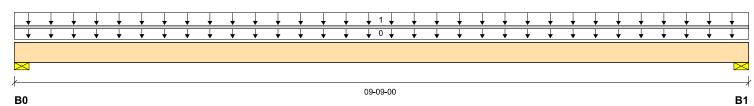
Job name: 45147 (4203)

Address: Pine Valley Description: First Floor Framing

City, Province, Postal Code: Vaughan, ON Builder: Gold Park

Gold Park Designer: NL

Code reports: CCMC 12472-R Company: Alpa Roof Trusses



### Total Horizontal Product Length = 09-09-00

Reaction Summary (Down / Uplift) (Ibs)

Reaction Sui	ililiary (Down / Op				
Bearing	Live	Dead	Snow	Wind	
B0, 3-1/2"	1024 / 0	413 / 0			
B1, 3-1/2"	1024 / 0	413 / 0			

Loa	Load Summary							Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	09-09-00	Тор		6			00-00-00
1		Unf. Area (Ib/ft²)	L	00-00-00	09-09-00	Top	40	15			05-03-00

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	4543 ft-Ibs	17696 ft <b>-I</b> bs	25.7%	1	04-10-08
End Shear	1513 lbs	7232 <b>l</b> bs	20.9%	1	01-03-06
Total Load Deflection	L/999 (0.101")	n\a	n\a	4	04-10-08
Live Load Deflection	L/999 (0.072")	n\a	n\a	5	04-10-08
Max Defl.	0.101"	n\a	n\a	4	04-10-08
Span / Depth	9.4				

Bearing	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	3-1/2" x 1-3/4"	2052 lbs	54.5%	27.5%	Spruce-Pine-Fir
B1	Wa <b>l</b> l/Plate	3-1/2" x 1-3/4"	2052 lbs	54.5%	27.5%	Spruce-Pine-Fir



### **Notes**

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4

# **Disclosure**

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SE007761



# Single 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

# B11 (Floor Beam)

Dry | 1 span | No cant.

**PASSED** 

March 19, 2020 14:32:52

**BC CALC® Member Report** 

**Build 7555** 

Code reports:

45147 (4203) Job name:

Pine Valley Address:

City, Province, Postal Code: Vaughan, ON Builder:

Gold Park

CCMC 12472-R

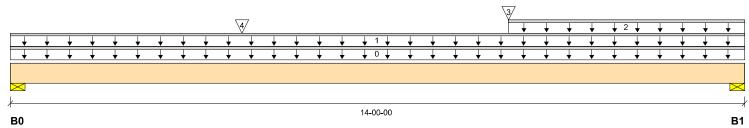
290672 File name:

Description: Specifier:

Designer: NL

Company: Alpa Roof Trusses

First Floor Framing



Total Horizontal Product Length = 14-00-00

Reaction Summary (Down / Uplift) (lbs)

**Bearing** Live Dead Snow Wind B0, 3-1/2" 872 / 0 828 / 0 B1, 3-1/2" 1146 / 0 954 / 0

Lo	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	14-00-00	Тор		6			00-00-00
1		Unf. Lin. (Ib/ft)	L	00-00-00	14-00-00	Top	27	74			n\a
2		Unf. Lin. (Ib/ft)	L	09-06-00	14-00-00	Тор	27	14			n\a
3		Conc. Pt. (lbs)	L	09-06-00	09-06-00	Тор	1024	413			n∖a
4		Conc. Pt. (lbs)	L	04-05-00	04-05-00	Тор	495	186			n∖a

		Factored	Demand/		
Controls Summary	Factored Demand	Resistance	Resistance	Case	Location
Pos. Moment	10431 ft <b>-I</b> bs	17696 ft <b>-I</b> bs	58.9%	1	09-06-00
End Shear	2658 lbs	7232 lbs	36.8%	1	12-08-10
Total Load Deflection	L/327 (0.497")	n\a	73.4%	4	07-01-06
Live Load Deflection	L/583 (0.279")	n\a	61.7%	5	07-03-05
Max Defl.	0.497"	n\a	49.7%	4	07-01-06
Span / Depth	13.7				

Bearing	յ Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	3-1/2" x 1-3/4"	2343 lbs	62.2%	31.4%	Spruce-Pine-Fir
B1	Wall/Plate	3-1/2" x 1-3/4"	2912 lbs	77.3%	39.0%	Spruce-Pine-Fir

# **Notes**

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4



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SE007762



**BC CALC® Member Report** 



# Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

B12 (Floor Beam) Dry | 1 span | No cant.

Specifier:

NL

March 19, 2020 14:32:52

**PASSED** 

**Build 7555** 

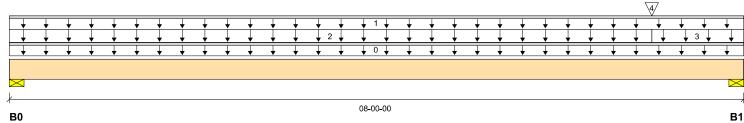
45147 (4203) Job name:

File name: 290672 Pine Valley First Floor Framing Address: Description:

City, Province, Postal Code: Vaughan, ON Builder:

Gold Park Designer:

Code reports: CCMC 12472-R Company: Alpa Roof Trusses



### Total Horizontal Product Length = 08-00-00

Reaction Summary (Down / Uplift) (lbs)

Wind **Bearing** Live Dead Snow B0, 3-1/2" 1338 / 0 619 / 0 B1, 3-1/2" 2075 / 0 1312 / 0

Lo	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (Ib/ft)	L	00-00-00	08-00-00	Тор		12			00-00-00
1		Unf. Lin. (Ib/ft)	L	00-00-00	08-00-00	Top	27	14			n\a
2		Unf. Area (lb/ft²)	L	00-00-00	07-00-00	Тор	40	15			07-00-00
3		Unf. Area (lb/ft²)	L	07-00-00	08-00-00	Top	40	15			02-03-00
4		Conc. Pt. (lbs)	L	07-00-00	07-00-00	Тор	1146	954			n∖a

		Factored	Demand/		
Controls Summary	Factored Demand	Resistance	Resistance	Case	Location
Pos. Moment	5568 ft-lbs	35392 ft <b>-l</b> bs	15.7%	1	04-05-02
End Shear	3780 lbs	14464 <b>I</b> bs	26.1%	1	06-08-10
Total Load Deflection	L/999 (0.042")	n\a	n\a	4	04-01-00
Live Load Deflection	L/999 (0.028")	n\a	n\a	5	04-01-00
Max Defl.	0.042"	n\a	n\a	4	04-01-00
Span / Depth	7.6				

Bearii	ng Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	3-1/2" x 3-1/2"	2780 lbs	36.9%	18.6%	Spruce-Pine-Fir
B1	Wall/Plate	3-1/2" x 3-1/2"	4752 lbs	63.1%	31.8%	Spruce-Pine-Fir



### **Notes**

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4

NAIL ONE PLY TO ANOTHER WITH 3-1/2" SPIRAL NAILS @ 10" STAGGERED IN 2 ROWS





# Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

# B14 (Floor Beam)

**PASSED** 

**BC CALC® Member Report** 

Dry | 2 spans | R cant.

March 19, 2020 14:32:52

**Build 7555** 

45147 (4203) Job name:

Pine Valley Address: Description: Second Floor Framing

City, Province, Postal Code: Vaughan, ON Builder:

Gold Park

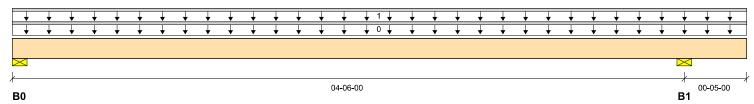
Code reports: CCMC 12472-R

290672 File name:

Specifier:

Designer: NL

Company: Alpa Roof Trusses



**Total Horizontal Product Length = 04-11-00** 

Reaction Summary (Down / Uplift) (lbs)

Live Dead B0, 3-1/2" 64 / 1 295 / 0 B1, 3-1/2" 69 / 0 324 / 0

Loa	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	04-11-00	Тор		12			00-00-00
1		Unf. Lin. (lb/ft)	L	00-00-00	04-11-00	Top	27	114			n∖a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	395 ft <b>-l</b> bs	23005 ft- <b>l</b> bs	1.7%	0	02-04-04
Neg. Moment	-15 ft- <b>l</b> bs	-23005 ft-lbs	n\a	0	04-06-00
End Shear	188 lbs	9401 lbs	2.0%	0	01-03-06
Cont. Shear	180 lbs	9401 lbs	1.9%	0	03-04-06
Total Load Deflection	L/999 (0.001")	n\a	n\a	9	02-04-04
Live Load Deflection	L/999 (0")	n\a	n\a	12	02-04-04
Total Neg. Defl.	2xL/1998 (-0")	n\a	n\a	9	04-11-00
Max Defl.	0.001"	n\a	n\a	9	02-04-04
Cant. Max Defl.	-0"	n\a	n\a	9	04-11-00
Span / Depth	4.3				



Bearing	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Wa <b>ll</b> /Plate	3-1/2" x 3-1/2"	414 lbs	8.4%	4.3%	Spruce-Pine-Fir
B1	Wall/Plate	3-1/2" x 3-1/2"	454 lbs	9.3%	4.7%	Spruce-Pine-Fir

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Design meets arbitrary (1") Cantilever Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4

Cantilevers require sheathed bottom flanges, blocking at cantilever support and closure at ends.

NAIL ONE PLY TO ANOTHER WITH 3-1/2" SPIRAL NAILS @ 6" O/C, STAGGERED IN 2 ROWS



**BC CALC® Member Report** 



# Quadruple 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

# B15 (Floor Beam)

Dry | 2 spans | L cant.

File name:

Specifier:

290672

March 19, 2020 14:32:52

**PASSED** 

**Build 7555** 

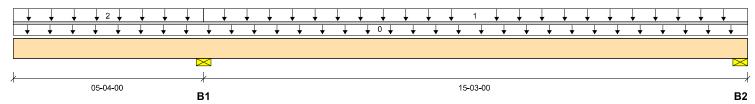
Job name: 45147 (4203)

Pine Valley Description: Address: Second Floor Framing

City, Province, Postal Code: Vaughan, ON

Builder: Gold Park Designer: NL

Code reports: CCMC 12472-R Company: Alpa Roof Trusses



**Total Horizontal Product Length = 20-07-00** 

Reaction Summary (Down / Uplift) (lbs)

Live Snow B1, 3-1/2" 5984 / 0 3324 / 0 B2, 3-1/2" 3405 / 404 1664 / 0

Lo	Load Summary							Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	20-07-00	Тор		24			00-00-00
1		Unf. Area (lb/ft²)	L	05-04-00	20-07-00	Top	40	20			11-00-00
2		Unf. Area (Ib/ft²)	L	00-00-00	05-04-00	Тор	40	20			10-08-00

<b>Controls Summary</b>	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	25148 ft <b>-l</b> bs	73615 ft <b>-l</b> bs	34.2%	3	13-01-00
Neg. Moment	-13323 ft- <b>l</b> bs	-73615 ft-lbs	18.1%	2	05-04-00
End Shear	5952 lbs	28927 lbs	20.6%	3	19-03-10
Cont. Shear	7039 lbs	28927 lbs	24.3%	1	06-05-10
Total Load Deflection	L/502 (0.359")	n\a	47.8%	10	12-10-10
Live Load Deflection	2xL/437 (-0.293")	n\a	82.4%	13	00-00-00
Total Neg. Defl.	2xL/361 (-0.355")	n\a	66.5%	10	00-00-00
Max Defl.	0.359"	n\a	35.9%	10	12-10-10
Cant. Max Defl.	-0.355"	n\a	35.5%	10	00-00-00
Span / Depth	15.2				



Bearing	յ Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B1	Wall/Plate	3-1/2" x 7"	13131 lbs	87.1%	43.9%	Spruce-Pine-Fir
B2	Wall/Plate	3-1/2" x 7"	7188 <b>I</b> bs	47.7%	24.0%	Spruce-Pine-Fir

## **Cautions**

Long Cantilever: Sheathing required on bottom flange and adjacent back span or bracing designed by the design professional of record. Design professional of record must address uplift at supports.

NAIL ONE PLY TO ANOTHER WITH 3-1/2" SPIRAL NAILS @ 12" O/C, STAGGERED IN 2 ROWS

PLUS SDW22634 SIMPSON WOOD SCREW @ 12" O/C, STAGGERED IN 2 ROWS



# Single 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

# B16 (Floor Beam)

Specifier:

290672

**PASSED** 

March 19, 2020 14:32:52

**BC CALC® Member Report** 

Dry | 1 span | No cant.

**Build 7555** 

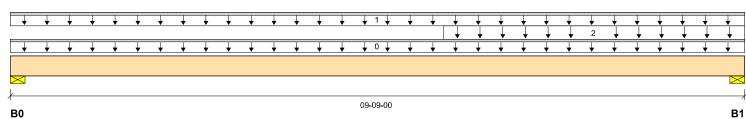
45147 (4203) Job name:

File name: Pine Valley Address: Description: Second Floor Framing

City, Province, Postal Code: Vaughan, ON

Builder: Gold Park Designer: NL

CCMC 12472-R Company: Code reports: Alpa Roof Trusses



### Total Horizontal Product Length = 09-09-00

Reaction Summary (Down / Uplift) (Ibs)

Bearing	Live	Dead	Snow	Wind
B0, 3-1/2"	254 / 0	143 / 0		
B1, 3-1/2"	649 / 0	292 / 0		

Loa	ad Summary						Live	Dead	Snow	Wind	Tributary
	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	09-09-00	Тор		6			00-00-00
1		Unf. Lin. (lb/ft)	L	00-00-00	09-09-00	Top	27	14			n∖a
2		Unf. Area (lb/ft²)	L	05-09-00	09-09-00	Top	40	15			04-00-00

		Factored	Demand/		
Controls Summary	Factored Demand	Resistance	Resistance	Case	Location
Pos. Moment	2056 ft- <b>I</b> bs	17696 ft <b>-I</b> bs	11.6%	1	06-03-02
End Shear	851 lbs	7232 <b>l</b> bs	11.8%	1	08-05-10
Total Load Deflection	L/999 (0.043")	n\a	n\a	4	05-02-05
Live Load Deflection	L/999 (0.029")	n\a	n\a	5	05-02-05
Max Defl.	0.043"	n\a	n\a	4	05-02-05
Span / Depth	9.4				

Beari	ng Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
В0	Wa <b>ll</b> /Plate	3-1/2" x 1-3/4"	560 lbs	14.9%	7.5%	Spruce-Pine-Fir
B1	Wall/Plate	3-1/2" x 1-3/4"	1338 <b>I</b> bs	35.5%	17.9%	Spruce-Pine-Fir

### **Notes**

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4



### **Disclosure**

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

BC CALC®, BC FRAMER®, AJS™. ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,

SE007766





# Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

# B17 (Floor Beam)

File name:

Specifier:

290672

**PASSED** 

**BC CALC® Member Report** 

Dry | 1 span | No cant.

March 19, 2020 14:32:52

**Build 7555** 

45147 (4203) Job name:

Pine Valley Address: Description: Second Floor Framing

City, Province, Postal Code: Vaughan, ON Builder:

Gold Park Designer: NL

Code reports: CCMC 12472-R Company: Alpa Roof Trusses

11-05-00 B0 В1

### Total Horizontal Product Length = 11-05-00

Reaction Summary (Down / Uplift) (lbs)

1604 / 0

**Bearing** Live Dead Snow Wind B0, 3" 1206 / 0 1592 / 0 B1, 3-1/2"

1215 / 0

Load Summary						Live	Dead	Snow	Wind	Tributary	
	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	11-05-00	Тор		12			00-00-00
1		Unf. Area (Ib/ft²)	L	00-00-00	11-05-00	Тор	40	20			07-00-00
2		Unf. Lin. (lb/ft)	L	00-00-00	11-05-00	Top		60			n∖a

<b>Controls Summary</b>	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	10361 ft <b>-l</b> bs	35392 ft <b>-l</b> bs	29.3%	1	05-08-04
End Shear	3047 lbs	14464 <b>I</b> bs	21.1%	1	01-02-14
Total Load Deflection	L/795 (0.166")	n\a	30.2%	4	05-08-04
Live Load Deflection	L/999 (0.094")	n\a	n\a	5	05-08-04
Max Defl.	0.166"	n\a	16.6%	4	05-08-04
Span / Depth	11.1				

Bearing	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	3" x 3-1/2"	3896 lbs	60.3%	30.4%	Spruce-Pine-Fir
B1	Wall/Plate	3-1/2" x 3-1/2"	3925 lbs	52.1%	26.3%	Spruce-Pine-Fir



### **Notes**

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4

NAIL ONE PLY TO ANOTHER WITH 3-1/2" SPIRAL NAILS @ 10" O/C. STAGGERED IN 2 ROWS





## Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

# B18 (Floor Beam)

File name:

Specifier:

290672

March 19, 2020 14:32:52

**PASSED** 

**BC CALC® Member Report** 

Dry | 1 span | No cant.

**Build 7555** 

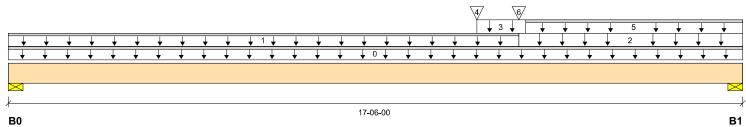
Job name: 45147 (4203)

Pine Valley Description: Address: Second Floor Framing

City, Province, Postal Code: Vaughan, ON Builder:

Gold Park Designer: NL

Code reports: CCMC 12472-R Company: Alpa Roof Trusses



#### Total Horizontal Product Length = 17-06-00

Poaction Summary (Down / Unlift) (lbs)

ixeaction out					
Bearing	Live	Dead	Snow	Wind	
B0, 3-1/2"	1074 / 0	756 / 0			
B1, 3-1/2"	2495 / 0	1615 / 0			

Load Summary						Live	Dead	Snow	Wind	Tributary
Tag Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0 Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	17-06-00	Тор		12			00-00-00
1	Unf. Lin. (Ib/ft)	L	00-00-00	12-02-00	Top	27	14			n∖a
2	Unf. Area (lb/ft²)	L	12-02-00	17-06-00	Top	40	20			03-06-00
3	Unf. Area (lb/ft²)	L	11-02-00	12-04-00	Top	40	15			10-08-00
4	Conc. Pt. (lbs)	L	11-02-00	11-02-00	Тор	254	143			n∖a
5	Unf. Lin. (lb/ft)	L	12-04-00	17-06-00	Top	27	14			n\a
6	Conc. Pt. (lbs)	L	12-02-00	12-02-00	Top	1604	1215			n∖a

		Factored	Demand/		
Controls Summary	<b>Factored Demand</b>	Resistance	Resistance	Case	Location
Pos. Moment	24132 ft- <b>l</b> bs	35392 ft <b>-l</b> bs	68.2%	1	12-02-00
End Shear	5287 lbs	14464 <b>I</b> bs	36.6%	1	16-02-10
Total Load Deflection	L/267 (0.767")	n\a	90.0%	4	09-07-07
Live Load Deflection	L/445 (0.459")	n\a	80.9%	5	09-07-07
Max Defl.	0.767"	n\a	76.7%	4	09-07-07
Span / Depth	17.2				

Bearing	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	3-1/2" x 3-1/2"	2556 lbs	33.9%	17.1%	Spruce-Pine-Fir
B1	Wall/Plate	3-1/2" x 3-1/2"	5762 lbs	76.5%	38.6%	Spruce-Pine-Fir



### **Notes**

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4

NAIL ONE PLY TO ANOTHER WITH 3-1/2" SPIRAL NAILS @ 12" O/C, STAGGERED IN 2 ROWS

SE007768





## Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

# B19 (Floor Beam)

Specifier:

Dry | 1 span | No cant.

March 19, 2020 14:32:52

**PASSED** 

**BC CALC® Member Report Build 7555** 

B1, 3-1/2"

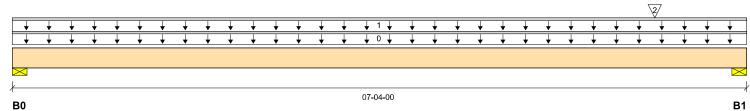
45147 (4203) Job name:

File name: 290672 Pine Valley Address: Description: Second Floor Framing

City, Province, Postal Code: Vaughan, ON

Gold Park Designer: NL

Builder: Code reports: CCMC 12472-R Company: Alpa Roof Trusses



#### Total Horizontal Product Length = 07-04-00

Reaction Summary (Down / Uplift) (Ibs)

2392 / 0

Bearing	Live	` ´ `Dead	Snow	Wi
B0, 3-1/2"	396 / 0	279 / 0		

1571 / 0

Lo	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	07-04-00	Тор		12			00-00-00
1		Unf. Lin. (Ib/ft)	L	00-00-00	07-04-00	Top	40	20			n∖a
2		Conc. Pt. (lbs)	L	06-05-00	06-05-00	Top	2495	1615			n∖a

		Factored	Demand/		
Controls Summary	Factored Demand	Resistance	Resistance	Case	Location
Pos. Moment	3777 ft <b>-I</b> bs	35392 ft <b>-l</b> bs	10.7%	1	06-05-00
End Shear	3301 lbs	14464 <b>I</b> bs	22.8%	1	06-00-10
Total Load Deflection	L/999 (0.019")	n\a	n\a	4	04-00-08
Live Load Deflection	L/999 (0.011")	n\a	n\a	5	04-01-06
Max Defl.	0.019"	n\a	n∖a	4	04-00-08
Snan / Denth	6.9				

Beari	ng Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	3-1/2" x 3-1/2"	943 lbs	12.5%	6.3%	Spruce-Pine-Fir
B1	Wall/Plate	3-1/2" x 3-1/2"	5552 lbs	73.7%	37.2%	Spruce-Pine-Fir



#### **Notes**

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4

NAIL ONE PLY TO ANOTHER WITH 3-1/2" SPIRAL NAILS @ 12" O/C, STAGGERED IN 2 ROWS



# Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP

# B20 (Floor Beam)

File name:

Specifier:

Designer:

290672

Wind

NL

**PASSED** 

March 19, 2020 14:32:52

**BC CALC® Member Report** 

Dry | 2 spans | No cant.

**Build 7555** 

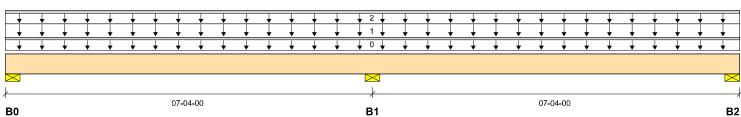
45147 (4203) Job name:

Address: Pine Valley Description: First Floor Framing

City, Province, Postal Code: Vaughan, ON Builder: Gold Park

Code reports:

CCMC 12472-R Company: Alpa Roof Trusses



Total Horizontal Product Length = 14-08-00

Snow

Reaction Summary (Down / Uplift) (Ibs)

Bearing	Live	Dead
B0, 3-1/2"	489 / 65	400 / 0
B1, 3-1/2"	1302 / 0	1227 / 0
B2, 3-1/2"	489 / 65	400 / 0

Lo	Load Summary						Live	Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	14-08-00	Тор		5			00-00-00
1		Unf. Area (lb/ft²)	L	00-00-00	14-08-00	Top	40	20			03-08-00
2		Unf. Lin. (lb/ft)	L	00-00-00	14-08-00	Top		60			n∖a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	1666 ft- <b>l</b> bs	11610 ft- <b>l</b> bs	14.3%	2	03-01-14
Neg. Moment	-2477 ft-Ibs	-11610 ft- <b>I</b> bs	21.3%	1	07-04-00
End Shear	808 lbs	5785 <b>I</b> bs	14.0%	2	01-01-00
Cont. Shear	1375 lbs	5785 <b>I</b> bs	23.8%	1	06-04-12
Total Load Deflection	L/999 (0.037")	n\a	n\a	9	03-04-14
Live Load Deflection	L/999 (0.024")	n\a	n\a	12	03-06-14
Total Neg. Defl.	L/999 (-0.004")	n\a	n\a	9	08-05-04
Max Defl.	0.037"	n\a	n\a	9	03-04-14
Span / Depth	9.0				

Bearing	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	3-1/2" x 1-3/4"	1234 lbs	32.7%	16.5%	Spruce-Pine-Fir
B1	Wall/Plate	3-1/2" x 1-3/4"	3487 lbs	92.5%	46.7%	Spruce-Pine-Fir
B2	Wall/Plate	3-1/2" x 1-3/4"	1234 lbs	32.7%	16.5%	Spruce-Pine-Fir

#### **Notes**

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4



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BC CALC®, BC FRAMER®, AJS™ ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,

SE007770



**BC CALC® Member Report** 



# Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

# PASSED

## **B21 (Floor Beam)**

File name:

Specifier:

290672

Dry | 1 span | No cant.

March 19, 2020 14:32:52

Build 7555

B1, 3-1/2"

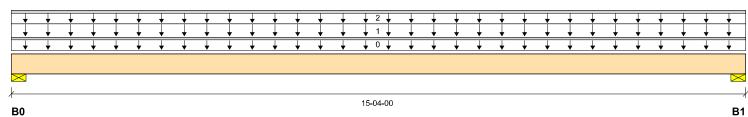
Job name: 45147 (4203)

Address: Pine Valley Description: Second Floor Framing

City, Province, Postal Code: Vaughan, ON

Builder: Gold Park Designer: NL

Code reports: CCMC 12472-R Company: Alpa Roof Trusses



#### Total Horizontal Product Length = 15-04-00

Reaction Summary (Down / Uplift) (Ibs)

2147 / 0

 Bearing
 Live
 Dead
 Snow
 Wind

 B0, 3-1/2"
 2147 / 0
 1626 / 0

1626 / 0

Loa	ad Summary						Live	Dead	Snow	Wind	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	15-04-00	Тор		12			00-00-00
1		Unf. Area (Ib/ft²)	L	00-00-00	15-04-00	Top	40	20			07-00-00
2		Unf. Lin. (lb/ft)	L	00-00-00	15-04-00	Тор		60			n∖a

<b>Controls Summary</b>	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	18947 ft <b>-I</b> bs	35392 ft <b>-l</b> bs	53.5%	1	07-08-00
End Shear	4374 lbs	14464 <b>I</b> bs	30.2%	1	01-03-06
Total Load Deflection	L/322 (0.555")	n\a	74.6%	4	07-08-00
Live Load Deflection	L/565 (0.316")	n\a	63.7%	5	07-08-00
Max Defl.	0.555"	n\a	55.5%	4	07-08-00
Span / Depth	15.0				

Beari	ing Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	3-1/2" x 3-1/2"	5252 lbs	69.7%	35.1%	Spruce-Pine-Fir
B1	Wall/Plate	3-1/2" x 3-1/2"	5252 lbs	69.7%	35.1%	Spruce-Pine-Fir



#### **Notes**

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 4

NAIL ONE PLY TO ANOTHER WITH 3-1/2" SPIRAL NAILS @ 10" O/C, STAGGERED IN 2 ROWS



# Maximum Floor Spans - M2.1, L/360

#### Design Criteria

Spans: Simple span

Live load = 40 psf and dead load = 20 psf
Deflection limits: L/360 under live load and L/240 under total load

Sheathing: 5/8 in. nailed-glued oriented strand board (OSB) sheathing



#### **Maximum Floor Spans**

			В	are		1/2 in. gypsum ceiling						
Joist depth	Joist series		On cent	re spacing			On cent	re spacing				
		12"	16"	19.2"	24"	12"	16"	19.2"	24"			
	NI-20	15'-1"	14'-3"	13'-10"	-	15'-7"	14'-9"	14'-3"	_			
0.4/0"	NI-40x	16'-2"	15'-3"	14'-8"	-	16'-7"	15'-8"	15'-1"	-			
9-1/2"	NI-60	16'-4"	15'-4"	14'-10"	-	16'-9"	15'-9"	15'-3"	-			
	NI-80	17'-3"	16'-3"	15'-8"	-	17'-8"	16'-7"	16'-0"	_			
	NI-20	17'-0"	16'-0"	15'-6"	-	17'-6"	16'-7"	16'-0"	_			
	NI-40x	18'-2"	17'-1"	16'-6"	-	18'-9"	17'-6"	16'-11"	_			
11-7/8"	NI-60	18'-5"	17'-3"	16'-8"	-	19'-0"	17'-8"	17'-1"	_			
	NI-80	19'-9"	18'-3"	17'-7"	-	20'-4"	18'-10"	18'-0"	-			
	NI-90	20'-2"	18'-8"	17'-10"	-	20'-9"	19'-2"	18'-4"	_			
	NI-40x	20'-1"	18'-8"	17'-10"	-	20'-10"	19'-4"	18'-6"	-			
14"	NI-60	20'-6"	18'-11"	18'-2"	-	21'-2"	19'-8"	18'-9"	_			
14	NI-80	21'-11"	20'-3"	19'-4"	-	22'-7"	20'-11"	20'-0"	_			
	NI-90	22'-5"	20'-8"	19'-9"	-	23'-0"	21'-4"	20'-4"	_			
	NI-60	22'-4"	20'-8"	19'-9"	-	23'-1"	21'-5"	20'-6"	_			
16"	NI-80	23'-11"	22'-1"	21'-1"	-	24'-8"	22'-10"	21'-9"	-			
	NI-90	24'-5"	22'-6"	21'-6"	_	25'-1"	23'-2"	22'-2"	_			

		Mi	d-span blocking	g with 1x4 inch s	trap	Mid-span blocking and 1/2 in. gypsum ceiling						
Joist depth	Joist series		On cent	re spacing			On cent	re spacing				
		12"	16"	19.2"	24"	12"	16"	19.2"	24"			
	NI-20	16'-11"	15'-5"	14'-6"	-	17'-1"	15'-5"	14'-6"	-			
0.4/0"	NI-40x	17'-11"	17'-0"	16'-5"	-	18'-5"	17'-4"	16'-7"	-			
9-1/2"	NI-60	18'-2"	17'-1"	16'-6"	-	18'-8"	17'-6"	16'-10"	-			
	NI-80	19'-5"	18'-0"	17'-5"	-	19'-10"	18'-5"	17'-8"	-			
	NI-20	19'-7"	18'-2"	17'-6"	-	20'-3"	18'-8"	17'-6"	-			
	NI-40x	21'-1"	19'-7"	18'-8"	-	21'-8"	20'-2"	19'-0"	-			
11-7/8"	NI-60	21'-4"	19'-9"	18'-11"	-	21'-11"	20'-5"	19'-6"	-			
	NI-80	22'-9"	21'-1"	20'-2"	-	23'-3"	21'-8"	20'-8"	-			
	NI-90	23'-3"	21'-6"	20'-6"	-	23'-9"	22'-0"	21'-0"	-			
	NI-40x	23'-8"	21'-11"	20'-11"	-	24'-4"	22'-8"	20'-11"	-			
14"	NI-60	24'-0"	22'-3"	21'-3"	-	24'-8"	22'-11"	21'-11"	-			
14	NI-80	25'-7"	23'-9"	22'-7"	-	26'-2"	24'-4"	23'-3"	-			
	NI-90	26'-1"	24'-2"	23'-0"	-	26'-8"	24'-9"	23'-7"	-			
	NI-60	26'-5"	24'-6"	23'-5"	-	27'-2"	25'-3"	24'-2"	-			
16"	NI-80	28'-2"	26'-1"	24'-10"	-	28'-10"	26'-9"	25'-6"	-			
	NI-90	28'-8"	26'-6"	25'-3"	_	29'-3"	27'-2"	25'-11"	_			

#### Notes

- 1. The tabulated clear spans are based on CSA O86-14 and NBC 2015, and are applicable to residential floor construction meeting the above design criteria.
- 2. For multiple-span applications, the end spans shall be 40% or more of the adjacent span.
- 3. Minimum bearing length shall be 1-3/4 inch for end bearings, and 3-1/2 inches for intermediate bearings.
- 4. Bearing stiffeners are not required when I-joists are used in accordance with this table, except as required for hangers.
- 5. Nordic I-joists are listed in CCMC Evaluation Report 13032-R and APA Product Report PR-L274C.



# Maximum Floor Spans - M4.1, L/360

#### Design Criteria

Spans: Simple span

Loads: Live load = 40 psf and dead load = 20 psf
Deflection limits: L/360 under live load and L/240 under total load

Sheathing: 3/4 in. nailed-glued oriented strand board (OSB) sheathing



#### **Maximum Floor Spans**

			В	are		1/2 in. gypsum ceiling						
Joist depth	Joist series		On cent	re spacing			On cent	re spacing				
		12"	16"	19.2"	24"	12"	16"	19.2"	24"			
	NI-20	15'-11"	15'-0"	14'-6"	13'-5"	16'-5"	15'-5"	14'-6"	13'-5"			
0.4/0"	NI-40x	17'-0"	16'-0"	15'-5"	14'-10"	17'-5"	16'-5"	15'-10"	14'-11'			
9-1/2"	NI-60	17'-2"	16'-2"	15'-7"	14'-11"	17'-7"	16'-7"	16'-0"	15'-4"			
	NI-80	18'-3"	17'-1"	16'-5"	15'-9"	18'-8"	17'-5"	16'-9"	16'-1"			
	NI-20	17'-11"	16'-11"	16'-3"	15'-8"	18'-7"	17'-5"	16'-10"	16'-1"			
	NI-40x	19'-4"	17'-11"	17'-3"	16'-7"	19'-11"	18'-6"	17'-9"	17'-0"			
11-7/8"	NI-60	19'-7"	18'-2"	17'-6"	16'-9"	20'-2"	18'-9"	17'-11"	17'-2"			
	NI-80	21'-1"	19'-6"	18'-6"	17'-7"	21'-7"	20'-0"	19'-0"	18'-0"			
	NI-90	21'-6"	19'-10"	18'-11"	17'-11"	22'-0"	20'-4"	19'-5"	18'-4"			
	NI-40x	21'-5"	19'-11"	18'-11"	18'-0"	22'-1"	20'-7"	19'-7"	18'-7"			
4.411	NI-60	21'-10"	20'-2"	19'-3"	18'-3"	22'-6"	20'-10"	19'-11"	18'-10			
14"	NI-80	23'-5"	21'-7"	20'-7"	19'-5"	24'-0"	22'-3"	21'-2"	20'-0"			
	NI-90	23'-10"	22'-1"	21'-0"	19'-10"	24'-5"	22'-7"	21'-6"	20'-4"			
	NI-60	23'-9"	22'-0"	21'-0"	19'-10"	24'-6"	22'-9"	21'-8"	20'-7"			
16"	NI-80	25'-6"	23'-7"	22'-5"	21'-2"	26'-2"	24'-3"	23'-1"	21'-10			
	NI-90	26'-0"	24'-0"	22'-10"	21'-6"	26'-7"	24'-8"	23'-5"	22'-2"			

		Mi	d-span blocking	g with 1x4 inch	strap	Mid-s	pan blocking an	d 1/2 in. gypsui	m ceiling		
Joist depth	Joist series		On cent	re spacing		On centre spacing					
		12"	16"	19.2"	24"	12"	16"	19.2"	24"		
	NI-20	17'-1"	15'-5"	14'-6"	13'-5"	17'-1"	15'-5"	14'-6"	13'-5"		
0.4/0"	NI-40x	18'-8"	17'-6"	16'-7"	14'-11"	19'-2"	17'-8"	16'-7"	14'-11"		
9-1/2"	NI-60	18'-11"	17'-8"	16'-10"	15'-7"	19'-5"	18'-0"	16'-10"	15'-7"		
	NI-80	20'-3"	18'-10"	17'-11"	17'-2"	20'-8"	19'-3"	18'-4"	17'-5"		
	NI-20	20'-3"	18'-8"	17'-6"	16'-1"	20'-7"	18'-8"	17'-6"	16'-1"		
	NI-40x	21'-10"	20'-4"	19'-0"	17'-0"	22'-5"	20'-10"	19'-0"	17'-0"		
11-7/8"	NI-60	22'-1"	20'-7"	19'-8"	18'-7"	22'-8"	21'-2"	20'-3"	18'-8"		
	NI-80	23'-8"	22'-0"	20'-11"	19'-10"	24'-1"	22'-6"	21'-6"	20'-4"		
	NI-90	24'-1"	22'-5"	21'-4"	20'-2"	24'-7"	22'-11"	21'-10"	20'-8"		
	NI-40x	24'-5"	22'-9"	20'-11"	18'-8"	25'-1"	22'-11"	20'-11"	18'-8"		
4.411	NI-60	24'-10"	23'-2"	22'-1"	20'-10"	25'-6"	23'-10"	22'-9"	21'-4"		
14"	NI-80	26'-6"	24'-8"	23'-6"	22'-2"	27'-1"	25'-3"	24'-1"	22'-9"		
	NI-90	27'-0"	25'-1"	23'-11"	22'-7"	27'-6"	25'-8"	24'-6"	23'-2"		
	NI-60	27'-3"	25'-5"	24'-3"	22'-11"	28'-0"	26'-2"	25'-0"	23'-1"		
16"	NI-80	29'-1"	27'-1"	25'-9"	24'-4"	29'-8"	27'-9"	26'-5"	25'-0"		
	NI-90	29'-7"	27'-6"	26'-2"	24'-9"	30'-2"	28'-2"	26'-10"	25'-5"		

#### Notes

- 1. The tabulated clear spans are based on CSA O86-14 and NBC 2015, and are applicable to residential floor construction meeting the above design criteria.
- 2. For multiple-span applications, the end spans shall be 40% or more of the adjacent span.
- 3. Minimum bearing length shall be 1-3/4 inch for end bearings, and 3-1/2 inches for intermediate bearings.
- 4. Bearing stiffeners are not required when I-joists are used in accordance with this table, except as required for hangers.
- 5. Nordic I-joists are listed in CCMC Evaluation Report 13032-R and APA Product Report PR-L274C.

The construction details for residential designs are prone to changes.

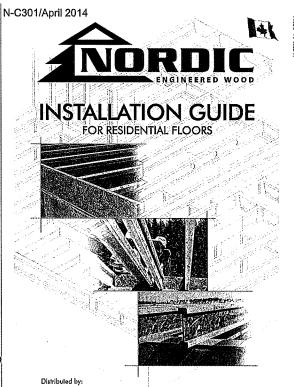
Details released after April 2014 supersedes N-C301

Installation must comply with latest documentation on I-Joist and other Nordic products from the http://nordic.ca/

This document does not constitute a record of the structural integrity of the building nor suitability of the design assumptions made. Nordic Structures is responsible only for the structural adequacy of its component based on the design criteria and loadings shown on the calculation sheets.

(Nordic Request 1810-095)





#### SAFETY AND CONSTRUCTION PRECAUTIONS





i-joists are not stable until completely installed, and will not carry any load until fully braced and sheafted.

Avoid Accidents by Following these Important Guidelines:

- Wolfd Actionins by rendering international moderation between the first plants, the In Broca and not also II-juist as it is intelled, within knoppers, blocking panels, rim board, and/or cross-bridging at joist ends. When I-juist are applied continuous over Intelor's supports and a local-bearing wall is planned at that location, blocking will be required at the Interior support.
- Whan the building is complated, the floor steathing will provide lateral support for the top flanges of the I-lots. Until this sheathing is applied, temporary bracing, office called struts, or temporary sheathing must be applied to prevent I-joist rollover or budding.
  - Bempartay President of successing.

    Bempartay President of sets and the last Inch minimum, at Issus 8 feet long and spaced no more than 8 feet on centre, and must be socured with a minimum of two 2-1/27 mails featured to the top participa of each felsion. Half the brocking to a lasted restrict in the end of each bay, Lap ends of adjoining bracking over all feats two lights.
- Or, sheathing (temporary or permanent) can be nailed to the top flange of the first 4 feet of 1-joints at the end of the bay.
- 3. For cantilevered I-joists, brace top and bottom flanges, and brace ends with closure panels, rim board, or cross-bridging.
- 4. Install and fully nail permanent sheathing to each I-joist before placing loads on the floor system. Then, stack building materials over beams or walls only.

5. Never Install a damaged Lipist.

proper storage or installation, kalkure to follow applicable building codes, kalkure to follow span tatings for orde: I-joists, failure to follow allowable hole sizes and locations, or failure to are web stifteners when required in result is realow accidents. Follow interes installation, guidalines corellolly.



- Store, stock, and handle t-joists vertically and level only.
- Always stack and handle Hjoists in the upright position only.
- 4. Do not store I-joists in direct contact with the ground and/or flatwise. 5. Protect I-juists from weather, and use spacers to separate bundles.
- 6. Bundled units should be kept intact until time of installation.
- When handling I-joists with a crone on the job site, take a few simple precautions to prevent damage to the I-joists and injury to your work crew.
  - ■Pick I-joists in bundles as shipped by the supplier.
  - "Orient the bundles so that the webs of the 1-joists are vertical.
  - $\bullet$  Pick the bundles at the 5% points, using a spreader bar if necessary.
- Do not handle l-joists in a horizontal orientation
- 9. NEVER USE OR TRY TO REPAIR A DAMAGED I-JOIST.



#### INSTALLING NORDIC I-JOISTS

- Before laying out floor system components, verify that I -joist flange widths treatch hunger widths. If not, contact your supplier.
- 2. Except for cutting to length, I-joist flanges should never be cut, drilled, or notched.
- 3. Install I-joists so that top and bottom flanges are within 1/2 inch of true vertical alignment
- 4. I-joints must be anchored securely to supports before floor shouthing is attached, and supports for multiple-spain joints must be level.
- 5. Minimum bearing lengths: 1-3/4 inches for end bearings and 3-1/2 inches for intermediate bearings.
- 6. When using hangers, seat I-joists firmly in hanger bottoms to minimize settlement.
- 7. Leave a 1/16-inch gap between the I-joist end and a header.
- 8. Concentrated loads greater than those first can normally be expected in residential construction should only be applied to the top surface of first loop fittings. Normal concentrated loads include track lighting fatures, auctio equipment and security conterars. Never supposed unaution of reacy loads from the 1-joil's cholonis fittings. Whenever possible, suspend off concentrated loads from the top of the 1-joils. Or, attach fals load to blocking that has been securely fastened to the 1-joils walbs.
- Never install Lights where they will be permonerally exposed to weather, or where they will remain in direct contact with controls or material.
- 10. Restrain ends of floor joists to prevent rollover. Use rim board, rim joists or I-joist blocking panels.
- 11. For I-joists installed over and beneath bearing walls, use full depth blocking panels, rim board, or squash blocks (cripple members) to transfer gravity loads through the floor system to the wall or foundation below.
- 12. Due to shrinkage, common framing lumber set on edge may nover be used as blocking or sim boards. I-joist blacking panels or other engineered wood products such as rim board must be cut to fit between the I-joists, and on I-joist-compatible depth relaceded.
- 13. Provide permonent lateral support of the bottom flange of all Lights at interior supports of multiple-span loists. Strailarly, support like bottom flange of all canflevered Lights of the end support need to the cantillover extension. In the completed structure, the gypsum wollboard calling provides this lateral support. Until the final finished ceiling is applied, temporary bracking or strots must be used.
- 14. If square-edge ponels are used, edges must be supported between I-joists with 2x4 blocking. Glue ponels to blocking to minimize squeeks. Blocking is not required under structural flaits flooring, such as wood strip flooring, or if a separate underlayment layer is fustalled.
- 15. Nail spacing: Space nails installed to the flange's top face in accordance with the applicable building code requirem approved building plans.

# TYPICAL NORDIC I-JOIST FLOOR FRAMING AND CONSTRUCTION DETAILS Figures 3, 4 or 5 Some framing requirements such as arection bracing and blocking panels have been omitted for clarity. tioles may be cut in web for plumbing, wiring and duct work. See Tables 1, 2 and Figure 7. $\Theta$ NOTE: Never cut or Nordic Lam or Structural (B)(B) ① 10 (1) 00000(1) (1)

All nails shown in the above datalls are assumed to be common wire nails unless otherwise noted. 3' (0.122' dis.) common spind rails may be substituted for 2-1/2' (0.126' dis.) common spind rails may be substituted for 2-1/2' (0.126' dis.) common with units. Training tumber assumed to be Spruce-Pino-Fir No. 2 or better, individual components not otherwin to scale for clarity.

(H)



Blocking Panel or Rim Joist	Maximum Factored Uniform Vertical Load* (plf)
NI Joists	3,300

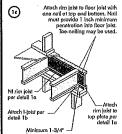
\*The uniform vertical load is limited to a joist depth of 16 inches or less and is based on standard term load duration is shall not be used in the design of a bending member, such as joist, header, or rafter. For concentrated vertical load transfer, see detail 1d.



- Attach rim board to top plate using 2-1/2\* wire or spiral toe-nails at 6" o.c To avoid splitting flange, start nails at loast 1-1/2\* from end of Ljoist. Nails avy be driven at an arryle to Minimum bearing length shall be 1-3/4" for the end bearings, and 3-1/2" for the intermediate bearings when applicable.

Maximum Factored Uniform Vortical Load\* (plf)

"The uniform vertical load is limited to a rim board depth of 16 inche or loss and is based on standard torm load duration. If shall not bused in the design of a bending member, such as joist, header, or ratios. For concentrated vertical load transfer, see detail 1 d.







Maximum Factored Vertical per Pair of Saugsh Blocks (lbs) 2x tumber 1-1/8\* Rim Board Plus 5,500 B,500 4,300 6,600

The construction details for residential designs are prone to changes.

Details released after April 2014 supersedes N-C301

Installation must comply with latest documentation on I-Joist and other Nordic products from the http://nordic.ca/

This document does not constitute a record of the structural integrity of the building nor suitability of the design assumptions made. Nordic Structures is responsible only for the structural adequacy of its component based on the design criteria and loadings shown on the calculation sheets.

(Nordic Request 1810-095)



#### N-C301/April 2014

### MAXIMUM FLOOR SPANS

- . Maximum cleur spans applicable to single-span or multiple-span residential floor construction with a design live load of 40 year for all deal and of 15 pst. The ultimate limit states are based on the factored loads of 1.50.L + 1.250. The serviceshilly limit states include the consideration for floor vibration and at live load deflection limit of 1/480. For multiple-span applications, the end spans shall be 40% or married the adjacent span.
- or more at the adjacent span.

  2. Spann are beared on a composite floor with glued-native distinct strend beared (158th sheething with a minimum shitchess of 5% flow find for losts spenting of 19.2 inches or lest, or 3/4 such for folist spacing of 24 inches. Adherive shall meet the requirements given in CQBS-11,26. Standard, No concrete topping or bridging element was assumed, Increased spans may be achieved with the used of gypsum and/or a row of blocking at mid-span.
- . Minimum bearing length shall be 1-3/4 inches for the end bearings, and 3-1/2 inches for the intermediate bearings.
- Bearing stiffeners are not required when 1-joists are used with the spans and spacings given in this table, except as required for hungers.
- This span chert is based on uniform loads. For applications with other than uniform loads, an engineering analysis may be required based on the use of the design properties.
- Tables are based on Limit States Design per CAN/CSA O86-09 Standard, and NBC 2010.
- 7. Si units conversion: 1 inch = 25.4 mm 1 foot = 0.305 m

# MAXIMUM FLOOR SPANS FOR NORDIC I-JOISTS SIMPLE AND MULTIPLE SPANS

	Joist		Simple	spons			Multiple spans						
Joist Depth	Series	50.00	On centre	spacing			On confro	spacing	44.				
		12"	16"	19.2	24"	12"	16"	19.2	24"				
Sec. 35.55	Nt-20	15-1	14'-2"	13'-9"	13-5	16'-3*	15-4'	14'-10"	14'-7'				
100	NI-40x	16-1*	15.2	14-8	14-9	17-5	16-5	15'-10"	15'-5'				
9-1/2"	NI-60	16'-3"	15'-4"	14'-10"	14'-11'	17.7	16'-7"	16'-0"	16'-6"				
1.00	N1-70	17:1"	16'-1"	15'-6"	15-7	18-7	17:4"	16'-9"	17-2				
S. 1966.	NI-80	17'-3"	16-3	15'-8"	15-9	18-10	171.6	16'-11"	17-5				
1000000	NI-20	16-11*	16'-0'	15'-5"	15-6"	18'-4"	17'-3"	16'-8'	16'-7"				
1.00	NI-40x	18-1*	17'-0"	16'-5"	16'-6"	20-0	18.6	17'-9"	17-7				
W 60.53	N1-60	18'-4"	17.3	16.7*	16-9	20'-3"	18.9	18'0'	18'-9'				
11-7/8	NI-70	19-6	18'-0"	17'-4"	17'-5"	21'-6"	19-11	19.0	19'-8'				
	NI 80	19'-9"	18'-3"	17-6*	17'-7"	21'-9"	20-2	19-3*	19-11*				
	NI-90	20-2*	18-7"	17-10	12-11*	22'-3'	20.7	19-8	19-9				
100	NI-90x	20'-4"	18-9	17-11*	18'-0"	22.5	20.9	19-10	20-5				
10.00	NI-40x	20'-1"	18-7	17'-10"	17:-11	22.2	20.6	19-8	19-4				
	NI-60	20'-5"	18-11	18-1	18-2	22-7*	20-11-	20-0	20-10				
14	NI-70	21'.7"	20.0	19-1	19-2	23-10	22-1	21-1	21'-10'				
14	NI-80	21'-11"	20'3	19-4	19-5	24'-3"	22.5	21'-5"	22-2				
3.73	NI-90	22-5	20'-8"	19-9	19-9'	24.9	22'-10"	21'-10"	21-10				
35.70	NI-90x	22.7	20-11*	19-11-	20-0	25'-0"	23'-1"	22-0	22.9				
72.5 33.5	NI-60	22-3	20.8	19-9	19-10*	24'-7"	22.9	21'-9'	22.9				
	NI-70	23.6	21'-9"	20.9	20-10	26'-0"	24'-0"	22-11	23.9				
16"	NI-80	23'-11"	22-1	21'-1"	21'-2"	26'-5'	24'-5"	23-3*	24-1				
45 8 73	NI-90	24'-5'	22.6	21'-5"	21'-6'	26'-11"	24'-10"	23-9	23.9				
14 F & 10	NI.90x	24'-8"	22.9	21.9.	21,10,	27'-3"	25-2	24.0	24'-10"				

# **1-JOIST HANGERS**

- 2. All nailing must meet the hanger manufacturer's recommendations.
- Hangers should be selected based on the joist depth, flange width and load capacity based on the maximum spans.
- . Web stilleners are required when the sides of the hangers do not laterally brace the top flange of the t-joist.





CCMC EVALUATION REPORT 13032-R

#### WEB STIFFENERS

#### RECOMMENDATIONS:

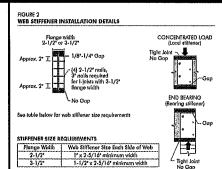
■ A bearing stiffener is required in all engineered applications with factored reactions greater than shown in the Hotel properties table found of the Hotel Construction Guide (C101). The gap between the stiffener and the flange is at the top.

A bearing stiffonor is required when the I-joist is supported in a hanger and the states of the hanger do not extend up to, and support, the top flange. The gap between the stiffener and flange is at the top.

sattener and flange is at the iop.

• A load stiffener is required at locations where a foctored concentrated load ground than 2,700 list is applied to the top flange between supports, or in the case of conditional conditions, or in the case of conditional conditions, anythere between the conditional conditions are supported by the condition of the condition of

Si units conversion: 1 inch = 25.4 mm



# NORDIC I-JOIST SERIES 5-P-F No.2 1950FMSR 2100FMSR 1950FMSR 33 pieces 33 pieces per unit per unit 23 pleass per unit 23 pieces per unit

Chanilers Chibougomau Ltd. harvests its own trees, which enables Nordic products to adhere to strict quality control procadures throughout the manufacturing process. Every phase of the operation, from forest to the finished product, reflects our commisment to quality.

Nordic Engineered Wood I-joists use only linger-jointed black spruce lumber in their flonges, ensuring consistent quality, superior strength, and longer spon corrying capacity.

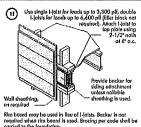


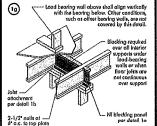
For nailing schedules for multiple beams, see the manufacturer's

Note: Unless hanger sides knorally support the top llange, bearing stiffeners shall be used.

①

Nordic Lam or SCL

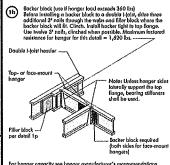




⑯

l-joist per detail 1b

Tight Joint No Gap



BACKER BLOCKS (Blocks must be long enough to permit required nailing without splitting)

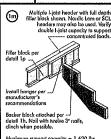
2-1/2" 1" 5-1/2"	Flange Width	Material Thickness Required*	Minimum Depth**					
	2-1/2*	1*	5-1/2"					
3-1/2* 1-1/2* 7-1/4*	3-1/2*	1-1/2*	7-1/4*					

Minimum grade for backer block material shall be S-F.F No. 2 or belter for solid saven lumber and wood structural panels conforming to CAN/CSA-0.437 Standard. For face-mount hangers use not joist depth minus 3-1/4" for joists with 1-1/2" thick flanges. For 2" thick flonges use net depth minus 4-1/4".



1/6" to 1/4" gap between top flange and filler block

®



⑽

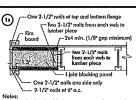
Maximum support capacity = 1,620 lbs

FILLER BLOCK REQUIREMENTS FOR DOUBLE I-JOIST CONSTRUCTION

℩

Note: Blocking required at bearing for lateral support, not shown for clarity.

Optional: Minimum 1x4 inch strap applied to underside of joist at blocking line or 1/2 inch minimum gypsum celling attached to underside of joists.



Notes:

In some local codes, blocking is prescriptively required in the first joint space (or first and second joint space) next to the starts joint. Where required, see local code requirement for spacing of the blocking.

All nails are common spiral in this detail.

- Support back of t-joist web during nailing to prevent damage to web/flange connection.
- Leave a 1/8 to 1/4-inch gap between top of filter block and bottom of top 1-joint
- for new books and solution to rep repair florage.

  Filler block is required between joists for foll length of span.

  Nati joists regarder with two rows of 3' onlise 10 greater on the control of 1' onlise 10 greater on the control of 1' onlise 10 greater of 1' onlise 10 greater of 1' onlise 1' on

3-1/2° x 1-1/2° 11-7/8 3-1/2° x

2-1/8" x 6" 2-1/6" x 8" 2-1/8" x 10" 2-1/8" x 12"

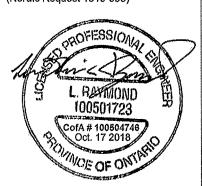
The construction details for residential designs are prone to changes.

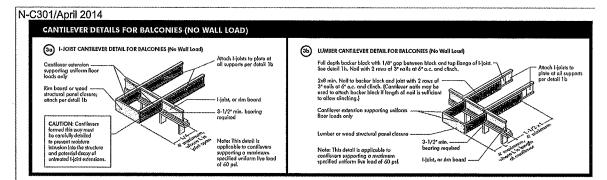
Details released after April 2014 supersedes N-C301

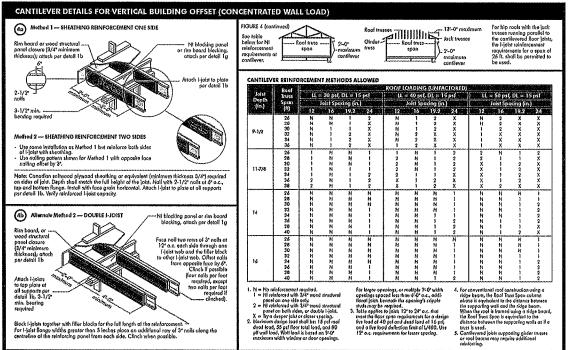
Installation must comply with latest documentation on I-Joist and other Nordic products from the http://nordic.ca/

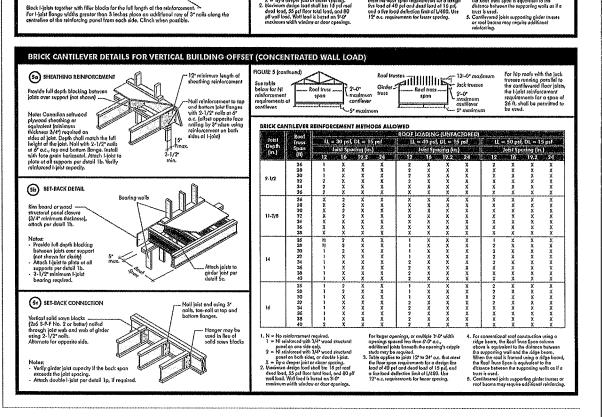
This document does not constitute a record of the structural integrity of the building nor suitability of the design assumptions made. Nordic Structures is responsible only for the structural adequacy of its component based on the design criteria and loadings shown on the calculation sheets.

(Nordic Request 1810-095)









The construction details for residential designs are prone to changes.

Details released after April 2014 supersedes N-C301

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(Nordic Request 1810-095)



### N-C301/April 2014

#### **WEB HOLES**

RULES FOR CUTTING HOLES AND DUCT CHASE OPENINGS:

- The distance between the inside edge of the support and the centreline of any hale or duct chase opening shall be in compliance with the requirements of Table 1 or 2, respectively.

  I-joint top and bottom flanges must NEVER be out, notched, or otherwise modified.
- 3. Whenever possible, field-cut holes should be centred on the middle of the web.
- Triburral (possible) amount failes strong to be tentined on the miscine of the vector. The maximum stap halo or the maximum depth of a duct chase populing that can be cut into an i-joist was stall equal the clear distance between the flanges of the i-joist minus 1/4 inch. A relatinum of 1/8 Inch, should always be maintained between the top or bottom of the halo or apening and the adjacent i-joist flange.
- The sides of square holes or longest sides of rectangular holes should not exceed 3/4 of the diameter of the maximum round hole permitted at that location.
- 3/4 of the diameter of the maximum round hole permitted at that facation.

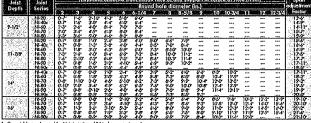
  4. Where rans a thom one hale is recessary, the distance a brusen edigicant hole edges shall exceed twice the diameter of the largest round hale or twice the size of the largest aware hale for rivice tile largest rectangular hale or dust clisse opening and each hole and duct chare opening that the sized and becated in compliance with the requirements of Tables 1 and 2, respectively.

  A kineckost is not considered a hole, may be utilized anywhere it occurs, and may be ignored for purposes of colcularing minimum distances between holes and/or duct chare openings.

  B. Holss resouring 1-1/2 hackes or smaller shall be paralleled onywhere in a conflictivated action of a joist. Holes of groofer size may be apprentited subject to verification.

- A 1-1/2 inch hole or smaller can be placed anywhere in the web provided that it
  meets the regularments of rule number 6 above.
- 10. All holes and duct chase openings shall be cut in a workman-like manner in accordance with the restrictions listed above and as illustrated in Figure 7.
- 11. Limit three maximum size holes per span, of which one may be a duct chase
- A group of round holes at approximately the same location shall be permitted if they meet the requirements for a single round halo circumscribed around them.

TABLE 1 LOCATION OF CIRCULAR HOLES IN JOIST WEBS Simple or Multiple Span for Dead Louds up to 15 psf and Live Loads up to 40 psf



Above table may be used for Lipids spacing of 24 inches an earlier or less. Holy location distance is measured from inside loce of supports to centre of licke Distances in this chart are based on uniformly looded joists.

The above table is based on the 1-joint used at their maximum span. If the 1-joints are placed at less than their full maximum span (see Maximum Floor Spans), the minimum distance from the centralized the late face of any support (D) as given above may be reduced as follows:

Oreduced in Sape In Dreduced in Oreduced in Distance from the inside lose of any appart to centre of hole, reduced for less shart maximum span applications (II). The reduced distance shall not be less than in subset from the late of the support to edge of the hole. The new part of the centre of the support of the subset in the subset in the subset in the subset of support (II). Span Adjustion Facility given in this table.

Span Adjustion Facility given in this table.

The meritum distance from the well-due of any support to centre of their from this table.

If agreed in genetic from 1, vice 1 in the observe colorables for "agreed."

FIGURE 7 FIELD-CUT HOLE LOCATOR

A knockout is NOT considered a hole, may be utilized wherever it occurs and may be ignored for purposes of calculating minimum distances between holes.



For reatingular holes, avoid over-cutting the comers, as this can couse unnessess stress concentrations. Slighth rounding the comers is recommended. Starting the comers is recommended. Starting the reatingular hole by diffilling a 1-Inch diamater hole in each of the four correr and them notificing the case between the holes is another good reathed to minimize damage to the 1-jobs.



com hibb may be used for hight spocing of 24 inches on centre of law.

The characterising location destores in measured from hidde loca of sepocits to centre of opening, as done which is broad on simple-upon points only. To other applications, control your local distributor, allower are located your local distributor, allower are located in undermy located local youth his memory for each requiremental for a design has located (AD pel and and all all all a pel and all all all all all and a law located distributors from all ADVIC for other applications, contact your local distributor.

#### INSTALLING THE GLUED FLOOR SYSTEM

- 1. Yips any mud, dirt, water, or ice from I-joint flanges before gluing.
- 2. Snap a chalk line across the t-joists four feet in from the wall for panel edge alignment and as a boundary for spreading glue.
- Spread only onough give to key one or two panels at a lime, or follow specific recommendations from the give manufacturer.
- Luy the first panel with tongue side to the wall, and noil in place. This protects the tongue of the next
  panel from damage when tapped into place with a block and sledgehammer.
- Apply a continuous line of glue (about 1/A-inch diarneter) to the top flange of a single I-joist. Apply glue in a winding pattern on wide areas, such as with double I-joists.
- 6. Apply two lines of give on Holass where panel ends but to assure proper gluing of each end.
  7. Altar the first row of panels is in place, spread give in the groove of one or two panels at a time before bying the east row. Often the marry to continuous or spaced, but good squeeze-out by applying at himmer line (10) linel) then used an Holass language.
- 8. Tap the second row of panels into place, using a block to protect groove edges.
- Stagger and joints in each succeeding row of panels. A 1/8-inch space between all end joints and 1/8 inch at 03 edges, including 18G9 edges, is recommended. (Use a specar tool or an 2-1/2" comm notil to surve accesses and constraint specing.)
- not to assure accurate and combitant spacing.)

  10. Compilate all nating of each panel before give sets. Check the manufacturer's recommendations for awe time. (Warm weather accelerates give setting.) Use 2' ring- or setter-shank rolls for panels 3/4-thet hibits or bass, and 2-1/2' ring- or setter-shank rolls for thickey ponols. Space notils per the table below. (Cases and Beparking may be required by some codes, or for disphagen construction. The flishind deck can be walked on right away and will carry construction loads without damage to the gibb band.

#### fasteners for sheathing and subflooring(1)

Maximum	Minimum		all Size and Ty	Maximum Spacing				
Joist Spaking (in.)	. Panel Thickness (in.)	Common King Thre Wire or Noils Spiral Nails or Screw		Skaples	edges Supports			
16	5/8	2'	1-3/4*	2'	6,	12'		
20	5/8	2'	1-3/4*	2'	6*	12'		
24	3/4	2'	1-3/4"	2"	6*	12'		

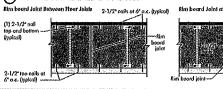
- 1. Fasteners of sheathing and subflooring shall conform to the above table.
- 2. Staples shall not be less than 1/16-inch in diarneter or thickness, with not less than a 3/8-inch crown driven with the crown parallel to faunting.
- 3. Flooring screys shall not be lass than 1/8-inch in diameter
- Special conditions may impose heavy traffic and concentrated loads that require construction in excess
  of the minimums shown.
- 5. Use only adherives conforming to CAN/COSE-71.26 Standard, Adherives for Field-Gluing Plywood to Lumber Framing for Floor System, applied in accordance with the manufacturer's recommendations. If OSB panels with socied surfaces and edges are to be used, use only solvent-based gives; check with panel manufactures.

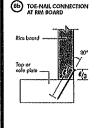
Ref.: NRC-CNRC, National Building Code of Canada 2010, Table 9.23.3.5.

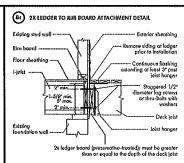
IMPORTANT NOTE:

Floor shouthing must be field glued to the I-joist flunges in order to achieve the maximum spans shown in this document. If sheathing is nailed only, I-joist spans must be verified with vary local distributor.

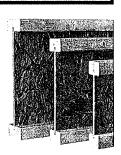
## RIM BOARD INSTALLATION DETAILS (8) ATTACHMENT DETAILS WHERE RIM BOARDS ABUT











1-1/2"

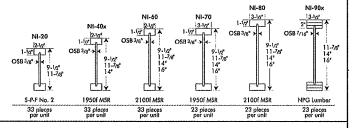
# **CONSTRUCTION DETAILS** FOR RESIDENTIAL FLOORS



www.nordicewp.com

Refer to the Installation Guide for Residential Floors for additional information.





#### WEB HOLE SPECIFICATIONS

CCMC EVALUATION REPORT 13032-R

RULES FOR CUTTING HOLES AND DUCT CHASE OPENINGS:

- The distance between the inside edge of the support and the controlline of any hole or duct chase opening shall be in compliance with the requirements of Tablet are 2, respectively.
   Helds to go and bettom langers must NEVER be cut, notched, or otherwise modified.
   Whenever possible, field-cut holes should be centred on the middle of the web.
   He make minum size hole or the maximum depth of a duct chase opaning that can be cut into an Helds when shall equal the clear distance between the flanges of the Helds into 114 lack. A mininum of 118 lack hall dawys be maintained between the top or bottom of the hole or opening and the adjacent Helds flange.
- 5. The sides of square holes or langest sides of rectangular holes should not exceed 3/4 of the diameter of the maximum round hole permitted at that location.
  6. Where more than one hole is necessary, the distance between adjacent hole edges shall acceed hive the diameter of the largest round hole or threa the size of the largest square hole for hive the file of the largest square hole for hive the largest hole or duct chave openingly and each hole and duct chave opening shall be sized and located in compliance with the requirements of Tables 1 and 2, respectively.
  7. A knockout is not considered a hole, may be vilized anywhere it occurs, and may be ignored for purposes of calculating minimum distances between holes and/or duct chase openings.
  8. Holes moesavring 1-1/2 Inches or smaller are permitted anywhere in a cantilevered section of a joist. Holes of greater size may be permitted subject to verification.

N-C303 / September 2013

- 9. A 1-1/2 Inch hole or smaller can be placed anywhere in the web provided that it meets the requirements of rule number 6 above.
  10. All holes and duct chase openings shall be cut in a overkmet with the restrictions listed above and as illustrated in Figure 7.

  11. Limit shree maximum size holes per span, of which one may be a duct chose opening.

  12. A group of round holes of approximately the same location shall be permitted if they meet the requirements for a single round hole accumscribed oround titem.

#### **LOCATION OF CIRCULAR HOLES IN JOIST WEBS**

Simple or Multiple Span for Dead Loads up to 15 psf and Live Loads up to 40 psf

4.4.4			N	inimun	Distar	ice fro	n Insid	e Face	of Any	Support	to Cer	nire of	Hole (ft	- in.)		
Joist Depth	Joist Series						Rou	nd Hol	e Dlam	eter (in.	)					
	201103	2	3	4	5	6	6-1/4	7	8	8-5/8	9	10	10-3/4	11	12	12-3/4
	NI-20	0'-7*	1'-6"	2'-10'	4'-3'	5'-8'	6'-0"		*	***			***	***		***
9-1/21	NI-40x	0:-7"	1'-6"	3'-0"	4'-4"	6'-0'	6'-4"	***	***	***			***	***	***	***
7-174	NI-60	1'-3'	2'-6"	4'-0"	5'-4"	7'-0'	7'-5"	***	***	***	***	***	***	***		***
	NI-70	2:0	3'-4"	4'-9*	6'-3"	8'-0"	8'-4"	***	***	***	***	***	***	***		***
	MI-80	2'-3'	3'-6"	5'-0'	6'-6"	8'-2"	8'-8'	***	***	***	***	***	-44	***		44.
	NI-20	0.7	0'-8"	1'0'	2'-4"	3'-8"	4'-0"	5'-0"	6'-6"	7'-9"		***	74>	***	***	***
	NI-40x	0'-7"	0'-8"	3'-3"	2'-8'	4'-0"	4-4	5'-5"	7'-0"	8'-4"		***	***	***	***	***
11-7/8	NI-60	0'-7"	1'-8"	3'-0"	4'-3"	5.9	6'-0"	7'-3"	8'-10"	10'-0"	***	***	***	400	***	***
	NI-70	1'-3"	2'-6"	4'-0"	5'-4"	6.9	7'-2"	8'-4"	10'-0"	11'-2"	***	***	***	***	***	***
	NI-80	1.6	2'-10"	4'-2"	5'-6"	7'-0'	7'-5'	8'-6"	10'-3"	11'-4"		***	***	***	***	***
	NI-90x	0'-7'	0.8.	0'-9"	2'-5'	4'-4"	4'-9"	6'-3"	***	***	7/7	***	***			***
	NI-40x	0.7	0'-8"	0'-8"	1'-0"	2'-4"	2'-9"	3'-9"	5'-2"	6'-0"	6'-6"	8'-3"	10'-2"	***	***	P44
14"	NI-60	0.7	0.84	1.8,	3'-0"	4'-3'	4'+8"	5'-8'	7'-2"	8'-0"	8'-8"	10.4	11:9"	***	***	F#4
14	NI-70	0.8	1'-10"	3'-0'	4-5	5'-10"	6'-2"	7'-3°	8'-9"	9'-9"	10'-4"	12'-0'	13'-5"	***	***	***
	NI-80	0.10	2'-0'	3'-4"	4'-9"	6.2	6'-5"	7'-6"	9'-0"	10'-0"	10'-8'	12'-4"	13'-9"	***	***	***
	NI-90x	0'-7"	0'-8'	0'-81	2'-0'	3'-9'	4'-2"	5'-5"	7'-3"	8'-5"	9'-2"			***		***
16'	MI-60	0.7'	0'-8"	0'-8"	1'-6"	2'-10'		41.2"	5'-6"	6'-4"	7'-0"	8'-5"	9'-8'	10-2	12'-2"	13'+9'
10.	NI-70	0.7	1'-0"	2'-3"	3'-6"	4'-10'		61.31	7'-8"	8'-6"	9'-2"	10'-8"	12'-0"		14'-0"	15'-6"
	NI-80	0'-7"	14-31	2-6°	3'-10"	5'-3'	5'-6"	6'-6"	8'-0"	9'-0'	9'-5"	11'-0"	1253	12'-9'	14'-5"	16'-0'
	NI-90x	0.7*	0'-8"	0.9	2'-0"	3'-6"	4'-0"	5'-0"	61.91	7'-9"	8'-4"	10'-2"	11'-6"	12'-0'	***	***

- 1. Above table may be used for 1-joint spacing of 24 inches on centra or less.
  2. Hole location distance is measured from inside face of supports to centre of hole.
  3. Distances in this chart are based on uniformly loaded joists.
  4. The above stable is based on the 1-joints bearing used at their maximum spans. The minimum distance as given above may be reduced for shorter spans; contact your local distributor.

## **DUCT CHASE OPENING SIZES AND LOCATIONS** Simple Span Only

Joist Depth	Joist Series	Minimum Distance from Inside Face of Supports to Centre of Opening (ft - in.)  Duct Chase Length (in.)								
		9-1/2*	NI-20 NI-40x NI-60 NI-70 NI-80	4'-1" 5'-3" 5'-4" 5'-1" 5'-3"	4'-5' 5'-8' 5'-9' 5'-5' 5'-8'	4'-10' 6'-0' 6'-2' 5'-10' 6'-0'	5'-4" 6'-5" 6'-7" 6'-3" 6'-5"	5'-8" 6'-10' 7'-1" 6'-7" 6'-10'	6'-1" 7'-3" 7'-5" 7'-1" 7'-3"	6'-6" 7'-8" 8'-0" 7'-6" 7'-8"
11-7/8*	NI-20 NI-40x NI-60 NI-70 NI-80 NI-90x	5-9' 6-8' 7-3' 7-1' 7-2' 7-7'	6'-2' 7'-2' 7'-8' 7'-4' 7'-7' 8'-1'	6'-6' 7'-6' 8'-0' 7'-9' 8'-0' 8'-5'	7'-1" 8'-1" 8'-6" 8'-6" 8'-5" 8'-5"	7'-5" 8'-6" 9'-0" 8'-7" 8'-10" 9'-4"	7'-9" 9'-1" 9'-3" 9'-3" 9'-3"	8'-3" 9'-6" 9'-9" 9'-6" 9'-8" 10'-2"	8'-9" 10'-1" 10'-3" 10'-1" 10'-2" 10'-8"	9'-4" 10'-9' 11'-0' 10'-4' 10'-8' 11'-2'
14°	NI-40x NI-60 NI-70 NI-80 NI-90x	8'-1* 8'-9' 8'-7' 9'-0' 9'-4'	8:-7' 9:-3' 9:-1' 9:-3'	9'-0' 9'-8' 9'-5' 10'-3'	9'-6" 10'-1" 9'-10" 10'-1" 10'-7"	10'-1" 10'-6" 10'-4" 10'-7"	10'-7' 11'-1' 10'-8' 11'-1'	11'-2' 11'-6' 11'-2' 11'-6' 12'-1'	12'-0' 13'-3' 11'-7' 12'-1' 12'-7'	12'-8' 13'-0' 12'-3' 12'-6' 13'-2'
16"	NI-60 NI-70 NI-80 NI-90x	10'-3' 10'-1' 10'-4' 11'-1'	10-8 10-5 10-9 11-5	11'-2' 11'-0' 11'-3' 11'-10'	11'-6" 11'-4" 11'-9" 12'-4"	12'-1° 11'-10' 12'-1° 12'-10'	12'-6' 12'-3' 12'-7' 13'-2'	13'-2" 12'-8" 13'-1" 13'-9"	14-1" 13-3" 13-8" 14-4"	14'-10' 14'-0' 14'-4' 15'-2'

- Above table may be used for I-joist spacing of 24 inches an centre or less.
   Duct chase opening location distance is measured from Inside face of supports to centre of opening.
   The above table is based on simple-span joists only. For other applications, contact your local distributor.
   Distances are based on uniformly loaded floor joists that meet the span requirements for a design live load of 40 ps and deed load of 15 pst, and a live load delication limit of I/480.
   The obove table is based on the I-joist being used at their maximum spans. The minimum distance as given above may be reduced for shorter spans; contact your local distributor.

#### FIGURE 7

#### FIELD-CUT HOLE LOCATOR

Duct chase opening (see Table 2 for minimum distance from bearing) 2x duct chase length or hole diameter. 2x diameter of larger hole or hole diamore, whichever is larger Maintain minimum 1/8" space between top and bottom flange --- all duct chose openings and holes



Knockouts are prescored holes provided for the contractor's convenience to install electrical or small plumbing lines. They are 1-1/2 inches in diameter, and are spaced 15 linches on canira along the length of the I-plats. Where possible, it is preferable to use knockouts instead of field-cut holes.

Never drill, cut or notch the flange, or over-cut the vieb.

Holes in webs should be cut with a sharp sow

For rectongular hales, avoid over-cutting the corners, as this can cause unnecessary stress concentrations. Slightly rounding the corners is recommended. Starling the rectongular hole by drilling a 1-inch diameter hole in each of the four corners and then naking the cuts between the holes is another good method to minimize damage to the L-joist.

#### SAFETY AND CONSTRUCTION PRECAUTIONS



WARNING: I-joists are not stable until completely installed, and will not carry any load until fully braced and sheathed. AVOID ACCIDENTS BY FOLLOWING THESE IMPORTANT GUIDELINES:

- AVOID ACCIDENTS BY FOLLOWING THESE MAPORTANT GUIDELINES:

  I Frace and nail each I-joist or is its installed, usign panages, blocking panels, tim board, and/or cross-bridging at joist ends. When I-joists are applied continuous over interior supports and a load-bearing wall is planned at that location, blocking will be required at the interior support.

  2. When the building is completed, the floor shealthing will provide lateral support for the top flonges of the I-joists. Until this shealthing is papiled, emporary bracting, often called situs, or temporary shealthing must be applied to prevent I-joist rollover or buckling.

  I Temporary bracing or struts must be 1x4 inch minimum, at least 8 feet long and spaced no more than 8 feet an centre, and must be secured with a minimum of two 2-1/2² noils fastened to the top surface of each I-joist. Noil the bracing to a lateral restraint at the end of each box, to pends of adjoining bracing over a least two I-joists, and it is not a lateral restraint at the end of each box, to pends of adjoining bracing over a least two I-joists at the end of the box.

  3. For contilevered I-joists, received pon and bottom flanges, and those ends with closure panels, rim board, or cross-bridging.

  4. Install and fully nail permanent shealthing to each I-joist before placing loads on the floor system. Then, stack building moderates over beams or worlds only.

  5. Never install a damaged I-joist.

Improper storage or installation, failure to follow applicable building codes, failure to follow span ratings for Nordic I-joists, failure to follow allowable hale sizes and locations, or failure to use web stiffaners when required can result in serious ocadents, Follow these installation guiddlines carefully.



#### **PRODUCT WARRANTY**

Chantiers Chibongaman guarantees that, in accordance with our specifications, Nordic products are free from manufacturing defects in material and workmanship.

Furthermore, Chantiers Chibongaman warrants that our products, then militeed in accordance with our bandling and installation instructions, will meet or exceed our specifications for the lifetime of the structure. 



The construction details for residential designs are prone to changes.

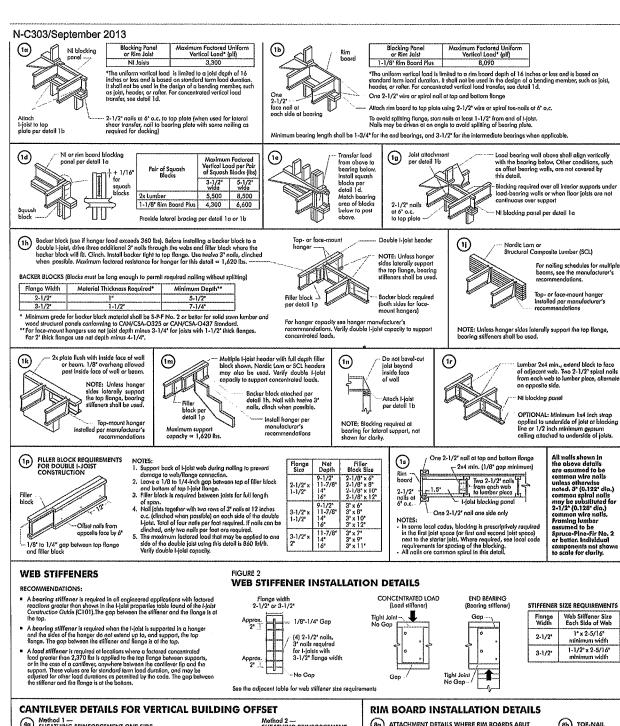
Details released after September 2013 supersedes N-303

Installation must comply with latest documentation on I-Joist and other Nordic products from the http://nordic.ca/

This document does not constitute a record of the structural integrity of the building nor suitability of the design assumptions made. Nordic Structures is responsible only for the structural adequacy of its component based on the design criteria and loadings shown on the calculation sheets.



(Nordic Request 1810-095)



STIFFENER SIZE REQUIREMENTS

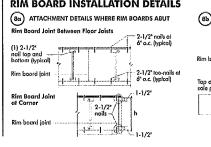
## Method 2 — SHEATHING REINFORCEMENT TWO SIDES 46 SHEATHING REINFORCEMENT ONE SIDE Rim board or wood structural panel closure (3/4\* minimum thickness); altach per detail 1b NI blocking panel or rim board blocking, atlach per detail 1g Uso same installation as Method 1 but reinforce both sides of I-joist with sheathing. Allach I-joist to plate per detail 1h pattern shows for Method 1 with opposite face nailing offset by 3\*. 2-1/2' nails 3-1/2" min. bearing required

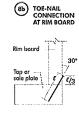
NOTE: Canadian softwood plywood shealthing or equivalent (minimum thickness 3/4") required on sides of joist. Depth shall match the full height of the joist. Natil with 2-1/2" acits at 6" a.c., top and bottom flange, install with face grain horizontal. Attack-light to plate of all supports per detail 15. Natil viri prindroad-light expectity. OROFESSIONAL BLANDONIS

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The construction details for residential designs are prone to changes.

Details released after September 2013 supersedes N-303

Installation must comply with latest documentation on 1-Joist and other Nordic products from the http://nordic.ca/

This document does not constitute a record of the structural integrity of the building nor suitability of the design assumptions made. Nordic Structures is responsible only for the structural adequacy of its component based on the design criteria and loadings shown on the calculation sheets.