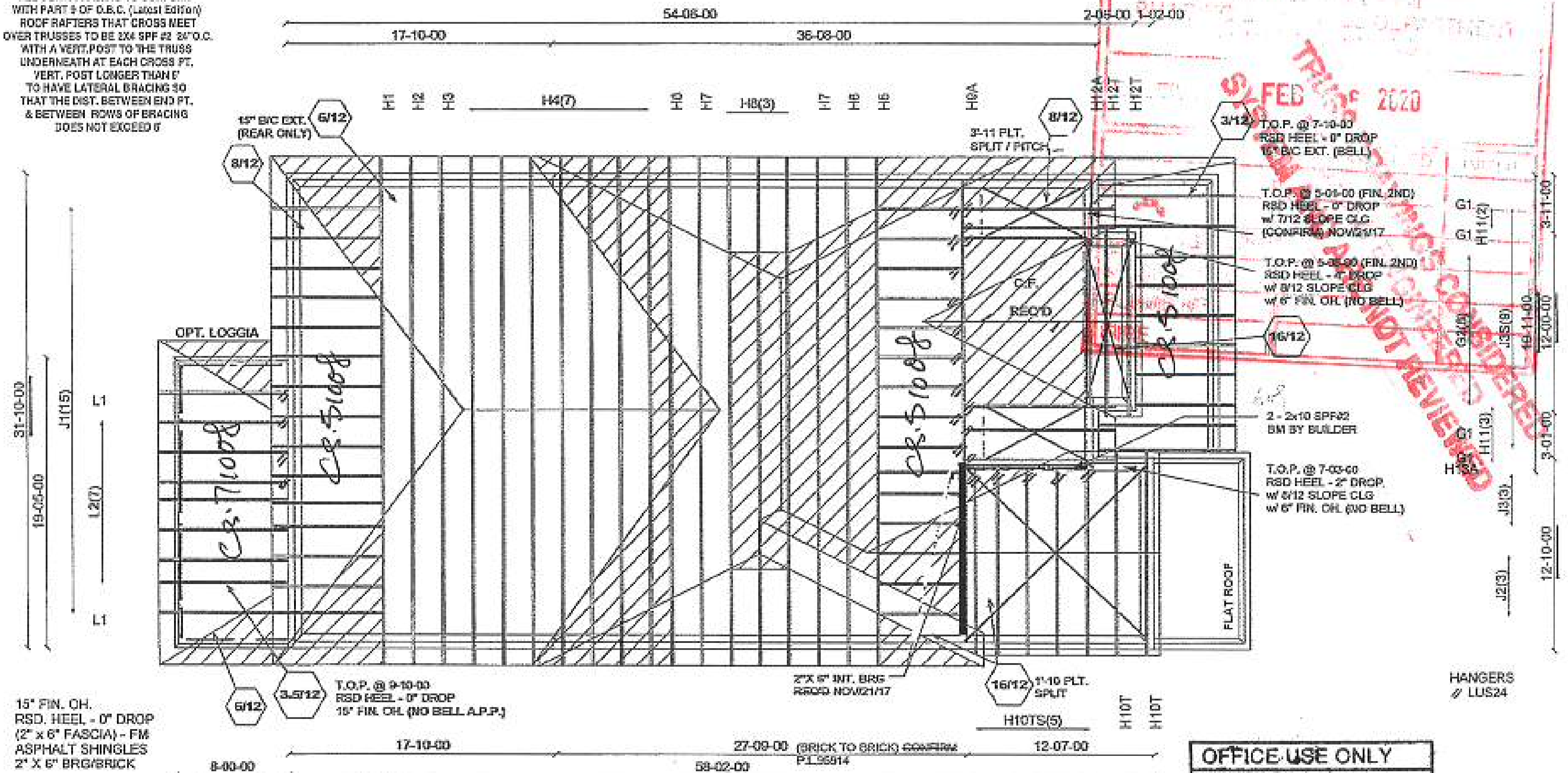
ALL CONV. FRAMING TO CONFORM
WITH PART 9 OF O.B.C. (Latest Edition)
ROOF RAFTERS THAT CROSS MEET
OVER TRUSSES TO BE 2X4 SPF #2 24"O.C.
WITH A VERT.POST TO THE TRUSS
UNDERNEATH AT EACH CROSS PT.
VERT. POST LONGER THAN 6'
TO HAVE LATERAL BRACING SO
THAT THE DIST. BETWEEN END PT.
& BETWEEN ROWS OF BRACING
DOES NOT EXCEED 6'



~~CONFIRM TRUSS LAYOUT/DIRECTION~~
P.L.95914

ALPA-BARRIE
FEB 2020
TRUSS GRAPHICS CONSIDERED
SYSTEM ... NOT REVIEWED

OFFICE USE ONLY	
ENGINEERED FROM	
A- 18023171	TO
A- 18023187	

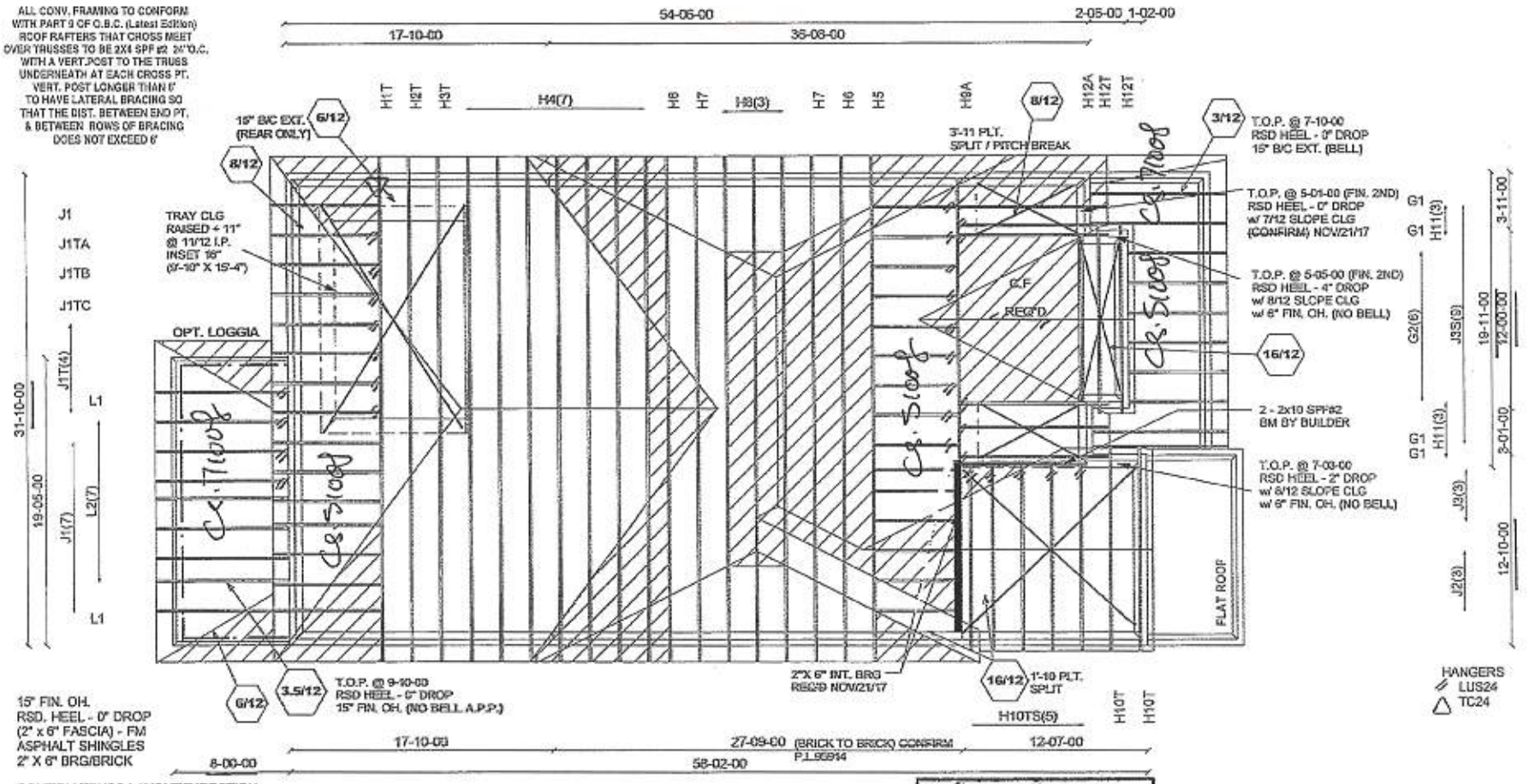
CONVENTIONAL FRAMING BY OTHERS

GLENARDEN
STD OR OPT LOGGIA

Job Track	45147	Builder / Location: GOLD PARK HOMES / VAUGHAN	Model / Elevation: 4003 REV1 / A
Layout ID:	292576	Project: PINE VALLEY	
Plan Log:	95914	Date: 1/19/2016 Designer: AMANDA	

RD-12005-GLD-OD-CM

ALPA-BARRIE



OFFICE USE ONLY
ENGINEERED FROM
A-1823174 TO
A-1823194
GLENARDEN

CONVENTIONAL FRAMING BY OTHERS

Job Track: 45147	Builder / Location: GOLD PARK HOMES / VAUGHAN	Model / Elevation: 4003 REV1	QR OPT LOGGIA / A OPT TRAY
Layout ID: 292577			
Plan Log: 95914	Project: PINE VALLEY		
	Date: 1/16/2018	Designer: AMANDA	

Stracon Engineering Inc.

69 Graydon Crescent
Richmond Hill, Ontario
L4B 3W7
(905) 832-2250 Fax (905) 832-0266

RESPONSIBILITIES

1. Stracon Engineering Inc. is responsible for the design of trusses as individual components.
2. It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities with jurisdictions.
3. All dimensions are to be verified by owner contractor, architect or other authority before manufacture.
4. Stracon Engineering Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Stracon Engineering Inc. drawings is specified for the truss as a component only and forms an integral part of the truss design.
5. It is the truss manufacture's responsibility to ensure that trusses are manufactured in conformance with Stracon Engineering Inc. specifications outlined below.

SPECIFICATIONS

1. Trusses designed by Stracon Engineering Inc. conforms to the relevant section of the Ontario Building Code of Canada (Part 9 or Part 4) or to the Canadian code for farm buildings, whichever applies to the building type indicated on the drawings as well as to the procedures established by the truss plate institute of Canada. Unit stresses used are as per CSA-086-09.
2. Lumber is to be the sizes and grade specified.
3. Moisture content of lumber is not to exceed 19% in service unless otherwise specified.
4. Lumber not to be treated with chemicals unless otherwise specified.
5. Plates shall be applied to both faces of the truss at each joint and shall be positioned as specified.
6. The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals not exceeding 12.5 times in thickness.
7. Where not rigid ceiling is attached directly to the bottom chord, laterally brace the chords at intervals not exceeding 3M (10') o.c.

January 15, 2014

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292576	H1	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 6.200 S Jan 6 2018 MiTek Industries, Inc. Thu Feb 15 14:06:53 2018 Page 1
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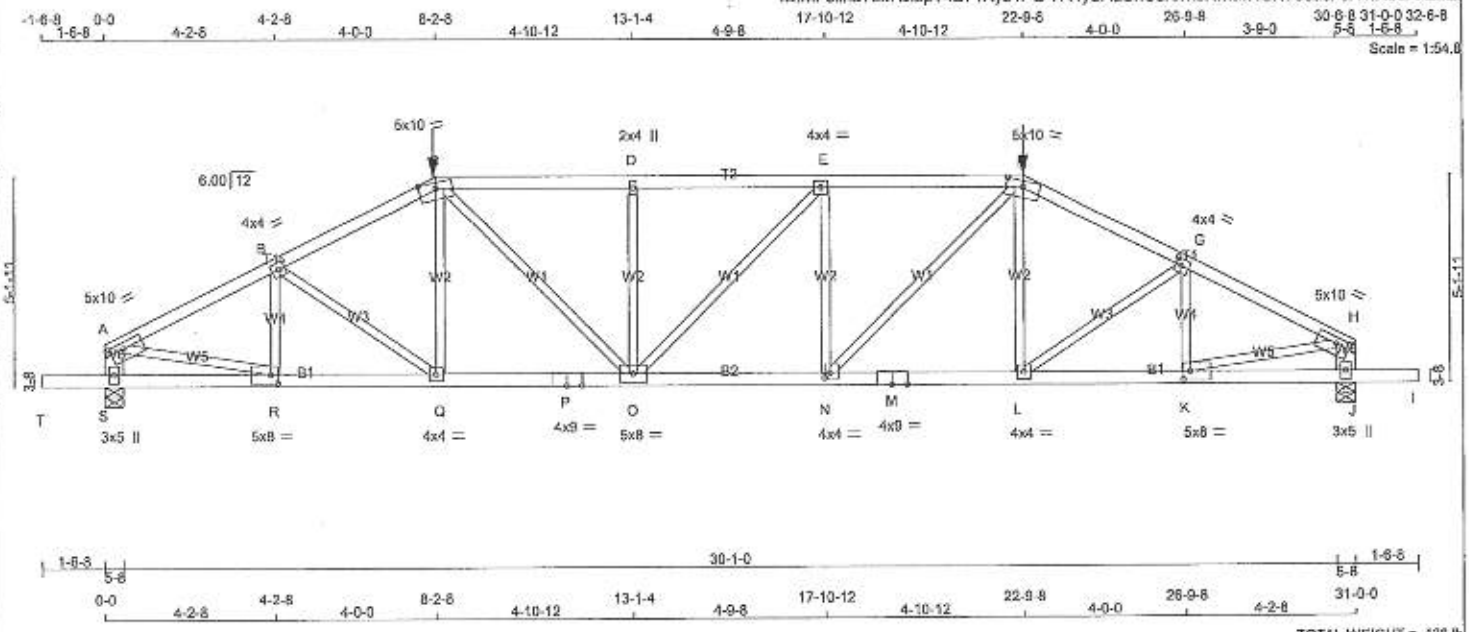

TOTAL WEIGHT = 126 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	1650F 1.5E	SPF
F - H	2x4	DRY	No.2	SPF
S - A	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
T - P	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-t	MT20	5.0	10.0	1.75	4.00
B	TMWW-t	MT20	4.0	4.0	2.00	1.75
C	TTWW-m	MT20	5.0	10.0	1.75	5.00
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTWW-m	MT20	5.0	10.0	1.75	5.00
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
H	TMWW-t	MT20	5.0	10.0	1.75	4.00
J	BMV1+p	MT20	3.0	5.0		
K	BMWW-t	MT20	5.0	8.0	2.50	2.00
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	4.0	9.0		
N	BMWW-t	MT20	4.0	4.0	1.50	1.50
O	BMWWW-t	MT20	5.0	8.0		
P	BS-t	MT20	4.0	9.0		
Q	BMWW-t	MT20	4.0	4.0		
R	BMWW-t	MT20	5.0	8.0	2.50	2.00
S	BMV1+p	MT20	3.0	5.0		

HANGERS NOTES
1)

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	RECMD BRG IN-SX
S	2939	0	2939	0	0	5-8	3-13
J	2939	0	2939	0	0	5-8	3-13

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	2099	1263 / 0	0 / 0	0 / 0	0 / 0	836 / 0	0 / 0
J	2099	1263 / 0	0 / 0	0 / 0	0 / 0	836 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.94 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)		FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX. CSI (L/C)	MAX. UNBRAC LENGTH
A- B	-4273 / 0		-78.0	-78.0	0.50 (1)	3.00
B- C	-4432 / 0		-78.0	-78.0	0.50 (1)	2.94
C- D	-4935 / 0		-153.5	-153.5	0.67 (1)	3.04
D- E	-4935 / 0		-153.5	-153.5	0.67 (1)	3.03
E- F	-4938 / 0		-153.5	-153.5	0.68 (1)	3.02
F- G	-4431 / 0		-78.0	-78.0	0.50 (1)	2.94
G- H	-4273 / 0		-78.0	-78.0	0.50 (1)	3.00
S- A	-2716 / 0		0.0	0.0	0.19 (1)	6.29
J- H	-2716 / 0		0.0	0.0	0.19 (1)	6.29
T- S	0 / 0		-96.5	-96.5	0.17 (1)	10.00
S- R	0 / 0		-36.4	-36.4	0.15 (4)	10.00
R- Q	0 / 3833		-36.4	-36.4	0.77 (1)	10.00
Q- P	0 / 3951		-36.4	-36.4	0.79 (1)	10.00
P- O	0 / 3951		-36.4	-36.4	0.79 (1)	10.00
O- N	0 / 4938		-36.4	-36.4	0.96 (1)	10.00
N- M	0 / 3951		-36.4	-36.4	0.79 (1)	10.00
M- L	0 / 3951		-36.4	-36.4	0.79 (1)	10.00
L- K	0 / 3833		-36.4	-36.4	0.77 (1)	10.00
K- J	0 / 0		-36.4	-36.4	0.15 (4)	10.00
J- I	0 / 0		-96.5	-96.5	0.17 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (L/C)
R- B	-522 / 0	0.10 (1)
B- Q	0 / 141	0.03 (1)
Q- C	0 / 184	0.07 (4)
C- O	0 / 1384	0.34 (1)
O- D	-801 / 0	0.32 (1)
O- E	-3 / 0	0.00 (1)
N- E	-802 / 0	0.32 (1)
N- F	0 / 1388	0.34 (1)
L- F	0 / 183	0.07 (4)
L- G	0 / 140	0.03 (1)
K- G	-522 / 0	0.10 (1)
A- R	0 / 3890	0.96 (1)
K- H	0 / 3890	0.96 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	8-2-8	-467	-467	—	FRONT	VERT	TOTAL
F	22-9-8	-467	-467	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: GPrimeHip
SIDE SETBACK = 8-2-8
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADOT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.23")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 817 (0.46")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.68/1.00 (E-F:1) , BC=0.96/1.00 (N-O:1) , WB=0.95/1.00 (H-K:1) , SSI=0.38/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	788	1987	1656

A-18023171

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292576	H2	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 8.230 S Jan 8 2018 MiTek Industries, Inc. Thu Feb 15 14:06:40 2018 Page 1
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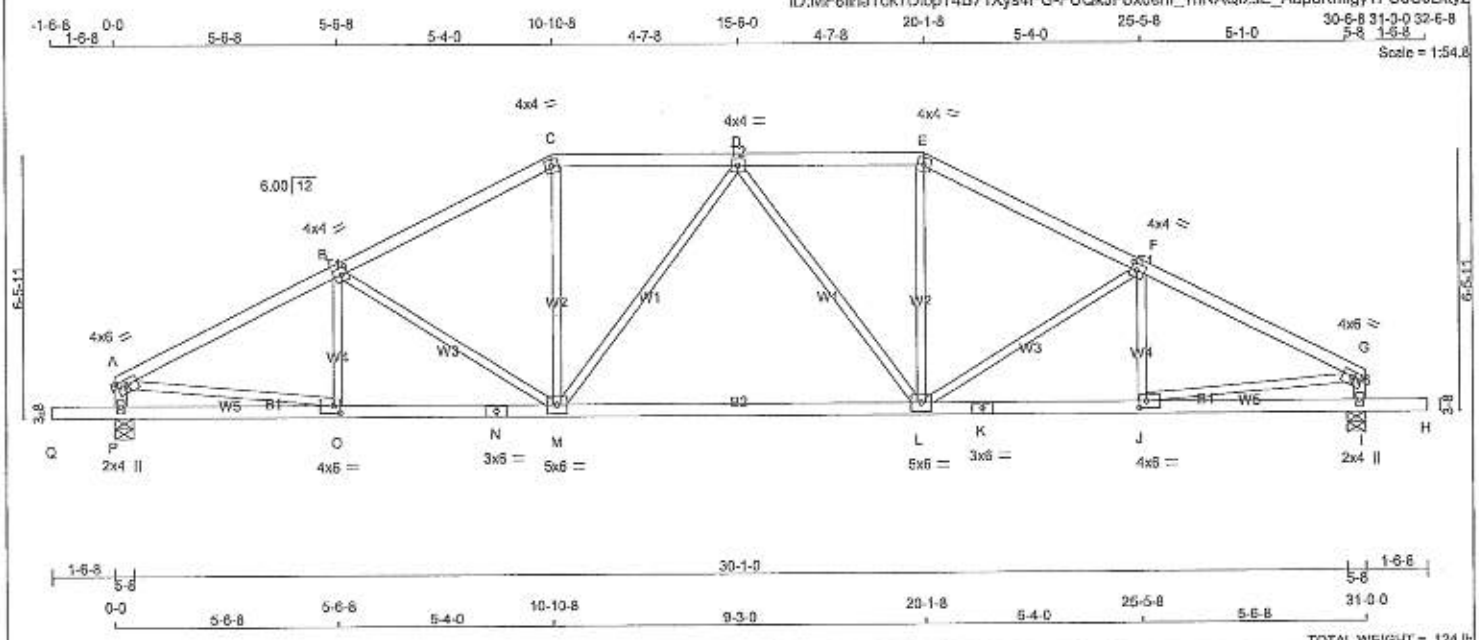

TOTAL WEIGHT = 124 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
P - A	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-t	MT20	4.0	6.0	1.50	2.75
B	TMWW-t	MT20	4.0	4.0	2.00	1.75
C	TTW-m	MT20	4.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0	2.00	1.75
G	TMWW-t	MT20	4.0	6.0	1.50	2.75
I	BMV1+p	MT20	2.0	4.0		
J	BMWW-t	MT20	4.0	6.0	2.00	2.00
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	5.0	6.0		
M	BMWWW-t	MT20	5.0	6.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	6.0	2.00	2.00
P	BMV1+p	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	1644	0	1644	0	0	5-8	2-7
I	1644	0	1644	0	0	5-8	2-7

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1172	715 / 0	0 / 0	0 / 0	0 / 0	457 / 0	0 / 0
I	1172	715 / 0	0 / 0	0 / 0	0 / 0	457 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.23 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	-2244 / 0	-78.0	-78.0	0.37 (1)		4.23
B-C	-1949 / 0	-78.0	-78.0	0.34 (1)		4.51
C-D	-1731 / 0	-78.0	-78.0	0.24 (1)		4.83
D-E	-1731 / 0	-78.0	-78.0	0.24 (1)		4.83
E-F	-1949 / 0	-78.0	-78.0	0.34 (1)		4.51
F-G	-2244 / 0	-78.0	-78.0	0.37 (1)		4.23
P-A	-1447 / 0	0.0	0.0	0.15 (1)		6.79
I-G	-1447 / 0	0.0	0.0	0.15 (1)		6.79
Q-P	0 / 0	-96.5	-96.5	0.16 (1)		10.00
P-O	0 / 0	-18.5	-18.5	0.13 (4)		10.00
O-N	0 / 2026	-18.5	-18.5	0.46 (1)		10.00
N-M	0 / 2026	-18.5	-18.5	0.46 (1)		10.00
M-L	0 / 1892	-18.5	-18.5	0.44 (1)		10.00
L-K	0 / 2026	-18.5	-18.5	0.46 (1)		10.00
K-J	0 / 2026	-18.5	-18.5	0.46 (1)		10.00
J-I	0 / 0	-18.5	-18.5	0.13 (4)		10.00
I-H	0 / 0	-96.5	-96.5	0.16 (1)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O-B	-176 / 4	0.04 (1)
B-M	-361 / 0	0.27 (1)
M-C	0 / 564	0.13 (1)
M-D	-271 / 0	0.32 (1)
D-L	-271 / 0	0.32 (1)
L-E	0 / 564	0.13 (1)
L-F	-361 / 0	0.27 (1)
J-F	-176 / 4	0.04 (1)
A-O	0 / 2043	0.46 (1)
J-G	0 / 2043	0.46 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.25")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.37/1.00 (F-G:1) , BC=0.46/1.00 (J-L:1) , WB=0.46/1.00 (G-J:1) , SSI=0.16/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (E) (INPUT = 0.90)
JSI METAL= 0.63 (N) (INPUT = 1.00)

A-18023172

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292576	H3	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Thu Feb 15 14:06:48 2018 Page 1
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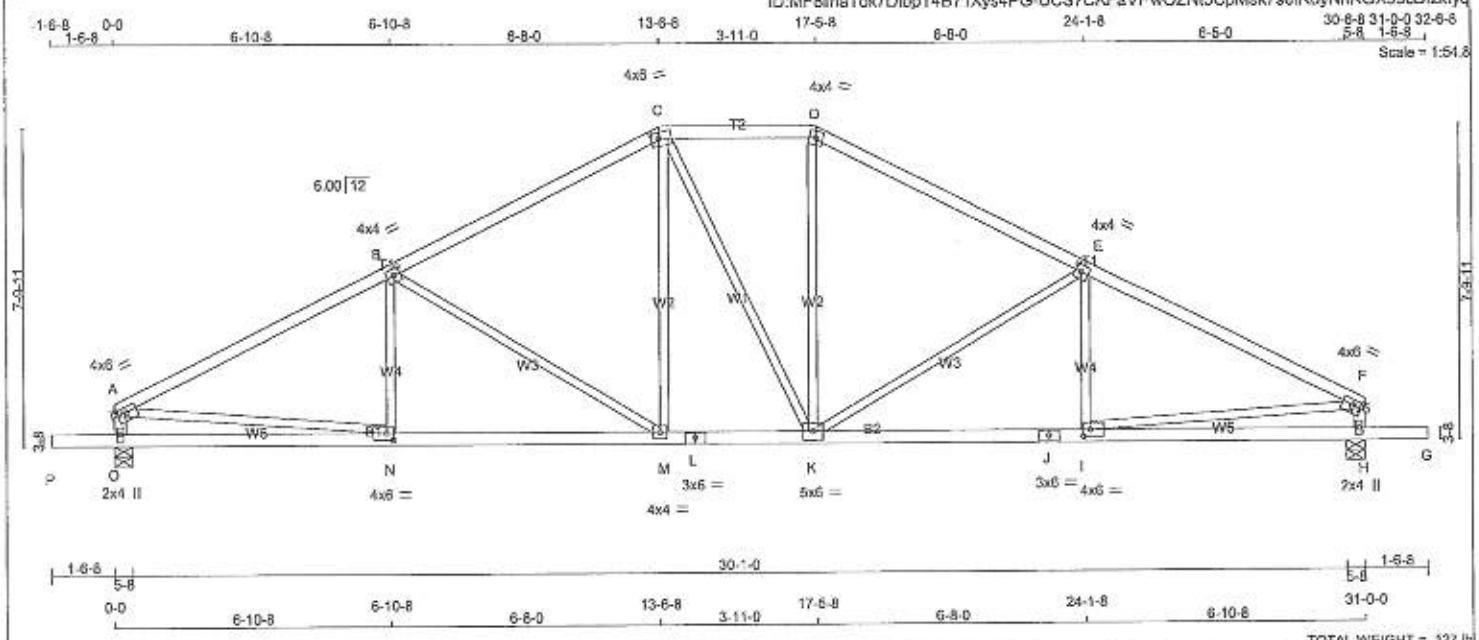

TOTAL WEIGHT = 127 lb (MPF)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
O - A	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
P - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-t	MT20	4.0	6.0	1.50	2.75
B	TMWW-t	MT20	4.0	4.0	2.00	1.75
C	TTWW-m	MT20	4.0	6.0	1.75	2.25
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.75
F	TMWW-t	MT20	4.0	6.0	1.50	2.75
H	BMV1+p	MT20	2.0	4.0		
I	BMWW-t	MT20	4.0	6.0	2.00	2.00
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	5.0	6.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMWW-t	MT20	4.0	6.0	2.00	2.00
O	BMV1+p	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1644	0	1644	0	0	5-8	2-7
H	1644	0	1644	0	0	5-8	2-7

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1172	715 / 0	0 / 0	0 / 0	0 / 0	457 / 0	0 / 0
H	1172	715 / 0	0 / 0	0 / 0	0 / 0	457 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.96 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-2254 / 0	-78.0	-78.0	0.61 (1)	3.96	
B- C	-1728 / 0	-78.0	-78.0	0.53 (1)	4.48	
C- D	-1526 / 0	-78.0	-78.0	0.18 (1)	5.15	
D- E	-1729 / 0	-78.0	-78.0	0.53 (1)	4.48	
E- F	-2254 / 0	-78.0	-78.0	0.61 (1)	3.96	
O- A	-1443 / 0	0.0	0.0	0.14 (1)	6.81	
H- F	-1442 / 0	0.0	0.0	0.14 (1)	6.81	
P- O	0 / 0	-96.5	-96.5	0.16 (1)	10.00	
O- N	0 / 0	-18.5	-18.5	0.21 (4)	10.00	
N- M	0 / 2041	-18.5	-18.5	0.43 (1)	10.00	
M- L	0 / 1525	-18.5	-18.5	0.31 (1)	10.00	
L- K	0 / 1525	-18.5	-18.5	0.31 (1)	10.00	
K- J	0 / 2041	-18.5	-18.5	0.43 (1)	10.00	
J- I	0 / 2041	-18.5	-18.5	0.43 (1)	10.00	
I- H	0 / 0	-18.5	-18.5	0.21 (4)	10.00	
H- G	0 / 0	-96.5	-96.5	0.16 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- B	-74 / 80	0.03 (4)
B- M	-513 / 0	0.81 (1)
M- C	0 / 411	0.09 (1)
C- K	0 / 2	0.00 (1)
K- D	0 / 413	0.09 (1)
K- E	-511 / 0	0.81 (1)
I- E	-76 / 78	0.03 (4)
A- N	0 / 2052	0.46 (1)
I- F	0 / 2052	0.46 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.18")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.61/1.00 (A-B:1) , BC=0.43/1.00 (M-N:1) , WB=0.81/1.00 (B-M:1) , SSI=0.23/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (N) (INPUT = 0.90)
JSI METAL= 0.63 (J) (INPUT = 1.00)

A-18023173

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292576	H4	7	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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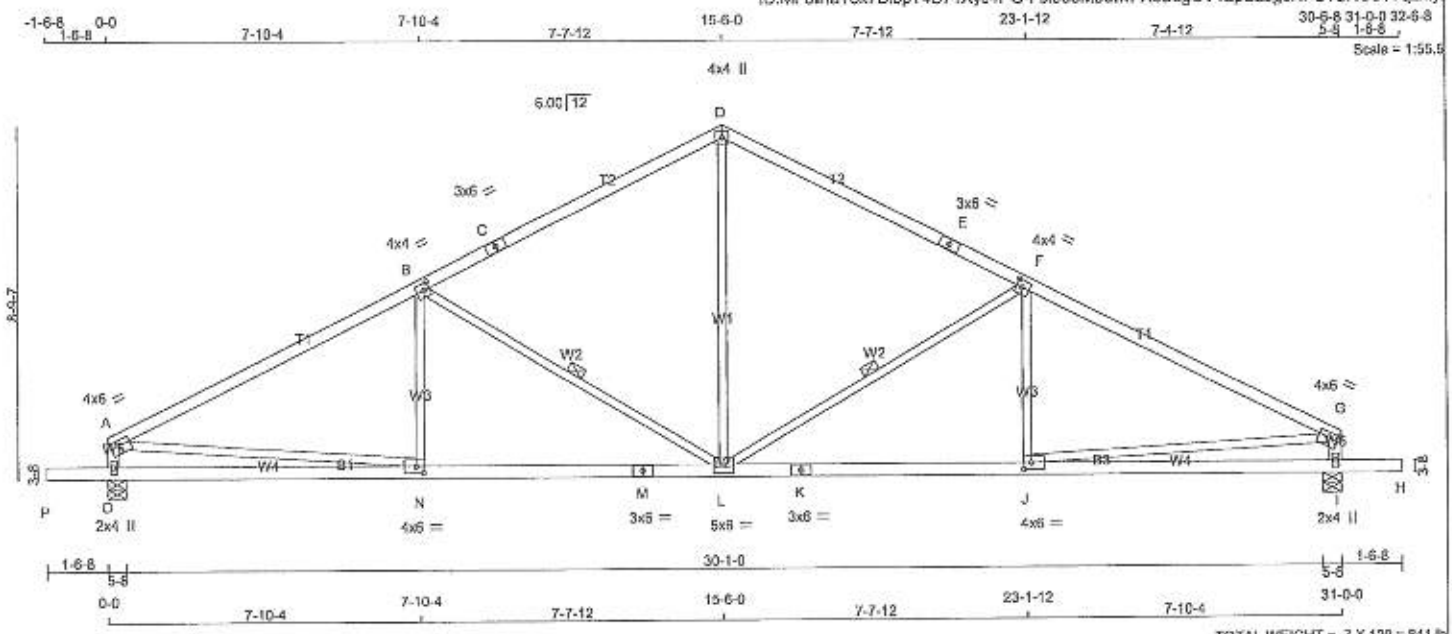

TOTAL WEIGHT = 7 X 120 = 841 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
O - A	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-t	MT20	4.0	6.0	1.50	2.75
B	TMWW-t	MT20	4.0	4.0	2.00	1.75
C	TS-t	MT20	3.0	6.0		
D	TTW+p	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0	2.00	1.75
G	TMWW-t	MT20	4.0	6.0	1.50	2.75
I	BMV1+p	MT20	2.0	4.0		
J	BMWW-t	MT20	4.0	6.0	2.00	2.25
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	5.0	6.0		
M	BS-t	MT20	3.0	6.0		
N	BMWW-t	MT20	4.0	6.0	2.00	2.25
O	BMV1+p	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1644	0	1644	0	0	5-8	2-7
I	1644	0	1644	0	0	5-8	2-7

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1172	715 / 0	0 / 0	0 / 0	0 / 0	457 / 0	0 / 0
I	1172	715 / 0	0 / 0	0 / 0	0 / 0	457 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.64 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-L, B-L

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-2233 / 0	-78.0	-78.0	0.63 (1)	3.64	L- D	0 / 928	0.21 (1)
B- C	-1572 / 0	-78.0	-78.0	0.70 (1)	4.33	L- F	-760 / 0	0.42 (1)
C- D	-1572 / 0	-78.0	-78.0	0.70 (1)	4.33	J- F	-30 / 109	0.04 (4)
D- E	-1572 / 0	-78.0	-78.0	0.70 (1)	4.33	B- L	-760 / 0	0.42 (1)
E- F	-1572 / 0	-78.0	-78.0	0.70 (1)	4.33	N- B	-30 / 109	0.04 (4)
F- G	-2233 / 0	-78.0	-78.0	0.63 (1)	3.64	A- N	0 / 2036	0.46 (1)
O- A	-1435 / 0	0.0	0.0	0.14 (1)	6.82	J- G	0 / 2036	0.46 (1)
I- G	-1435 / 0	0.0	0.0	0.14 (1)	6.82			
P- O	0 / 0	-96.5	-96.5	0.16 (1)	10.00			
O- N	0 / 0	-18.5	-18.5	0.26 (4)	10.00			
N- M	0 / 2026	-18.5	-18.5	0.46 (1)	10.00			
M- L	0 / 2026	-18.5	-18.5	0.46 (1)	10.00			
L- K	0 / 2026	-18.5	-18.5	0.46 (1)	10.00			
K- J	0 / 2026	-18.5	-18.5	0.46 (1)	10.00			
J- I	0 / 0	-18.5	-18.5	0.26 (4)	10.00			
I- H	0 / 0	-96.5	-96.5	0.16 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.63/1.00 (F-G:1) , BC=0.46/1.00 (J-L:1) , WB=0.46/1.00 (G-J:1) , SSI=0.26/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	619	354	1667	788	1997	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (N) (INPUT = 0.90)
JSI METAL= 0.62 (M) (INPUT = 1.00)

A-18023174

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292576	H5	1	1	TRUSS DESC.	

Alpe Roof Truss, Maple

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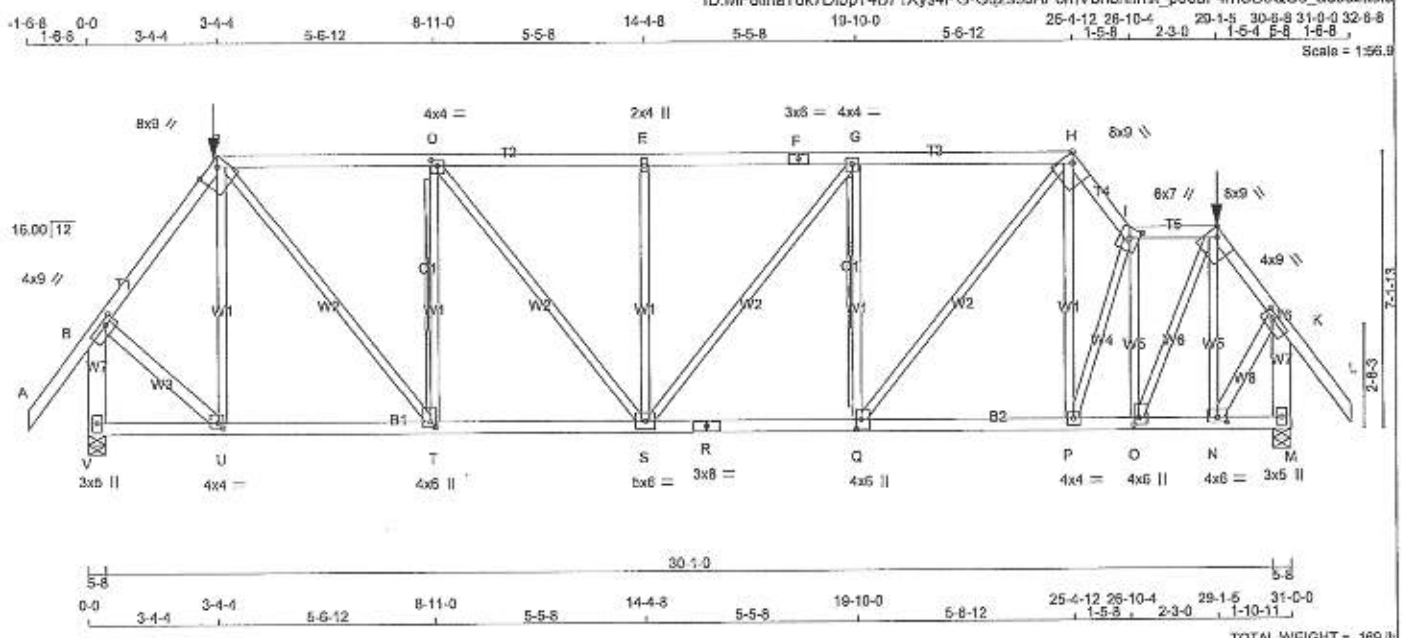

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1-6-6 1-5-8 1-5-4 1-6-8

LUMBER
N. I. G. A. RULES

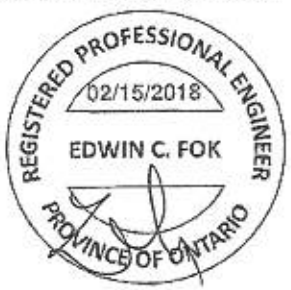
CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	1650F 1.5E	SPF
F - H	2x4	DRY	1650F 1.5E	SPF
H - I	2x4	DRY	No.2	SPF
I - J	2x4	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
V - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
V - R	2x4	DRY	No.2	SPF
R - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
B - U	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	9.0	1.50	3.00
C	TTWW-h	MT20	8.0	9.0	2.00	6.25
D	TMVW-t	MT20	4.0	4.0	1.75	2.00
E	TMV+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMVW-t	MT20	4.0	4.0		
H	TTWW-h	MT20	8.0	9.0	Edge	2.75
I	TTWW+m	MT20	8.0	7.0	3.00	2.75
J	TTWW-h	MT20	8.0	9.0	Edge	2.75
K	TMVW-t	MT20	4.0	9.0	1.50	3.00
M	BMV1+p	MT20	3.0	5.0		
N	BMWW-t	MT20	4.0	6.0	1.50	3.00
O	BMWW+t	MT20	4.0	6.0	2.00	1.50
P	BMWW-t	MT20	4.0	4.0		
Q	BMWW+t	MT20	4.0	6.0	3.00	1.50
R	BS-t	MT20	3.0	9.0		
S	BMWWW-t	MT20	5.0	8.0		
T	BMWW+t	MT20	4.0	6.0	1.50	1.50
U	BMWW-t	MT20	4.0	4.0	1.50	1.50
V	BMV1+p	MT20	3.0	5.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	3002	0	3002	0	0	5-8	4-4
M	3046	0	3046	0	0	5-8	4-6

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	2139	1313 / 0	0 / 0	0 / 0	0 / 0	827 / 0	0 / 0
M	2169	1336 / 0	0 / 0	0 / 0	0 / 0	834 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.31 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT D-T, G-Q

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 52	-78.0	-78.0	17 (1)	10.00	
B-C	-2256 / 0	-78.0	-78.0	28 (1)	4.29	
C-D	-2829 / 0	-153.5	-153.5	77 (1)	3.64	
D-E	-3377 / 0	-153.5	-153.5	81 (1)	3.31	
E-F	-3377 / 0	-153.5	-153.5	81 (1)	3.31	
F-G	-3377 / 0	-153.5	-153.5	81 (1)	3.31	
G-H	-3158 / 0	-153.5	-153.5	80 (1)	3.43	
H-I	-3304 / 0	-153.5	-153.5	70 (1)	3.67	
I-J	-2107 / 0	-153.5	-153.5	20 (1)	4.45	
J-K	-1924 / 0	-78.0	-78.0	22 (1)	4.61	
K-L	0 / 52	-78.0	-78.0	17 (1)	10.00	
V-B	-2934 / 0	0.0	0.0	32 (1)	6.07	
M-K	-3052 / 0	0.0	0.0	33 (1)	5.99	
V-U	0 / 0	-38.4	-38.4	20 (4)	10.00	
U-T	0 / 1334	-38.4	-38.4	38 (4)	10.00	
T-S	0 / 2830	-38.4	-38.4	63 (1)	10.00	
S-R	0 / 3159	-38.4	-38.4	70 (1)	10.00	
R-Q	0 / 3159	-38.4	-38.4	70 (1)	10.00	
Q-P	0 / 1994	-38.4	-38.4	50 (1)	10.00	
P-O	0 / 2158	-38.4	-38.4	48 (1)	10.00	
O-N	0 / 1090	-38.4	-38.4	32 (1)	10.00	
N-M	0 / 0	-38.4	-38.4	13 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
U-C	-705 / 0	0.64 (1)
C-T	0 / 2365	0.59 (1)
T-D	-1629 / 0	0.60 (1)
D-S	0 / 878	0.22 (1)
S-E	-771 / 0	0.70 (1)
S-G	0 / 350	0.09 (1)
Q-G	-1213 / 0	0.45 (1)
Q-H	0 / 1841	0.46 (1)
P-H	0 / 738	0.18 (1)
P-I	-874 / 0	0.26 (1)
O-I	-2382 / 0	0.99 (1)
O-J	0 / 2520	0.62 (1)
N-J	-1060 / 0	0.45 (1)
B-U	0 / 1613	0.29 (1)
N-K	0 / 1696	0.30 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE
C	3-4-4	-191	-191	—	FRONT	VERT	TOTAL
J	29-1-5	-107	-107	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CHL	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CHL	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 3-4-4
RIGHT SETBACK = 1-10-11
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDTL LOADS BASED ON 55 % OF GSL

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.28")

CSI: TC=0.81/1.00 (F-G:1) , BC=0.70/1.00 (Q-S:1) , WB=0.99/1.00 (I-O:1) , SSI=0.44/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1657	788	1987	1056

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

A-18023175

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292576	H6	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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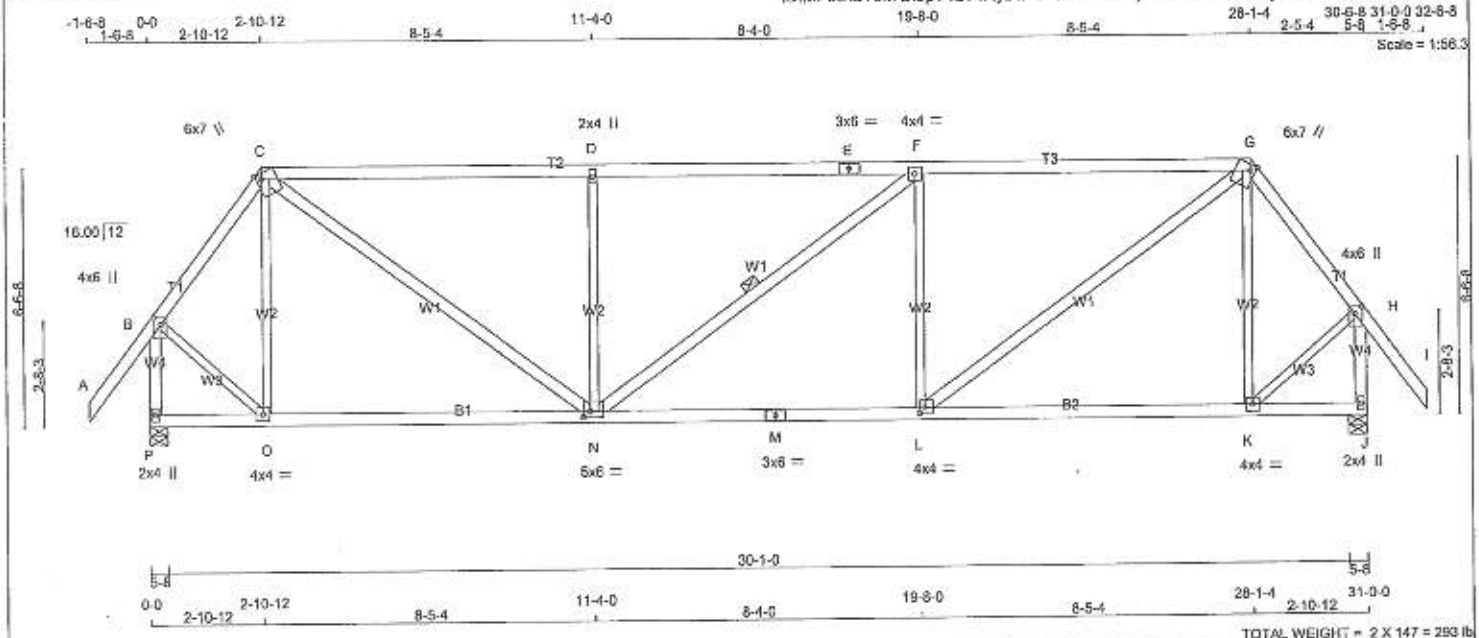

TOTAL WEIGHT = 2 X 147 = 293 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	1650F 1.5E	SPF
E - G	2x4	DRY	1650F 1.5E	SPF
G - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.25
C	TTWW+m	MT20	6.0	7.0	Edge	1.50
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTWW+m	MT20	6.0	7.0	Edge	1.50
H	TMVW+p	MT20	4.0	6.0	2.00	2.25
J	BMV1+p	MT20	2.0	4.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMWW-t	MT20	4.0	4.0	1.50	1.50
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	5.0	6.0	1.75	2.00
O	BMWW-t	MT20	4.0	4.0		
P	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	1625	0	1625	0	0	5-8	2-6
J	1625	0	1625	0	0	5-8	2-6

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1158	720 / 0	0 / 0	0 / 0	0 / 0	435 / 0	0 / 0
J	1156	720 / 0	0 / 0	0 / 0	0 / 0	435 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.47 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 52	-78.0	-78.0	0.15 (1)	10.00
B- C	-1099 / 0	-78.0	-78.0	0.13 (1)	5.89
C- D	-1823 / 0	-78.0	-78.0	0.82 (1)	4.48
D- E	-1823 / 0	-78.0	-78.0	0.81 (1)	4.47
E- F	-1823 / 0	-78.0	-78.0	0.81 (1)	4.47
F- G	-1824 / 0	-78.0	-78.0	0.82 (1)	4.47
G- H	-1099 / 0	-78.0	-78.0	0.13 (1)	5.89
H- I	0 / 52	-78.0	-78.0	0.15 (1)	10.00
P- B	-1625 / 0	0.0	0.0	0.23 (1)	6.49
J- H	-1625 / 0	0.0	0.0	0.23 (1)	6.49
P- O	0 / 0	-18.5	-18.5	0.19 (4)	10.00
O- N	0 / 648	-18.5	-18.5	0.32 (4)	10.00
N- M	0 / 1824	-18.5	-18.5	0.44 (1)	10.00
M- L	0 / 1824	-18.5	-18.5	0.44 (1)	10.00
L- K	0 / 649	-18.5	-18.5	0.32 (4)	10.00
K- J	0 / 0	-18.5	-18.5	0.19 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- C	-368 / 0	0.26 (1)
C- N	0 / 1458	0.23 (1)
N- D	-707 / 0	0.49 (1)
N- F	0 / 0	0.00 (1)
L- F	-707 / 0	0.49 (1)
L- G	0 / 1460	0.23 (1)
K- G	-370 / 0	0.26 (1)
B- O	0 / 823	0.19 (1)
K- H	0 / 823	0.19 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.82/1.00 (F-G:1) , BC=0.44/1.00 (L-N:1) , WB=0.49/1.00 (F-I:1) , SSI=0.31/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (N) (INPUT = 0.90)
JSI METAL= 0.65 (M) (INPUT = 1.00)

A-18023176

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292576	H7	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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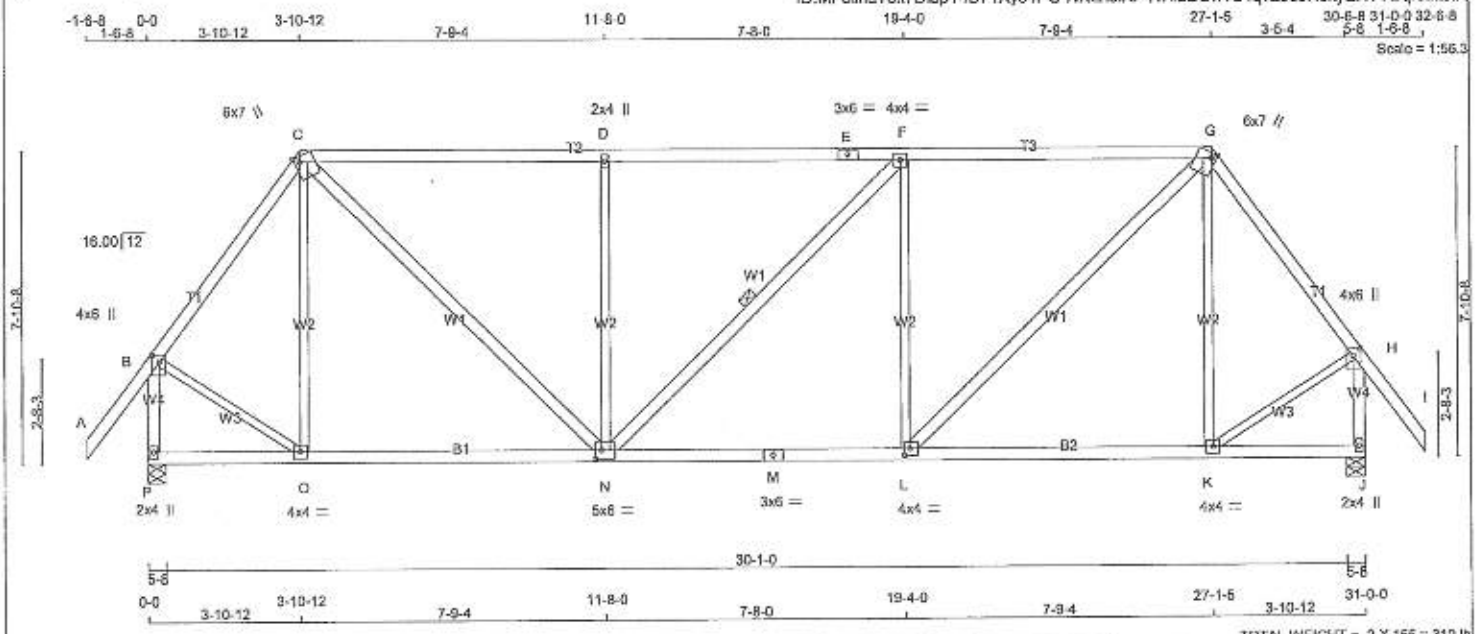

TOTAL WEIGHT = 2 X 155 = 310 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF
L - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW+p	MT20	4.0	6.0	2.00	2.25
C	TTWW+m	MT20	6.0	7.0	Edge	1.50
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTWW+m	MT20	6.0	7.0	Edge	1.50
H	TMW+p	MT20	4.0	6.0	2.00	2.25
J	BMV1+p	MT20	2.0	4.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMWW-t	MT20	4.0	4.0	2.00	1.50
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	5.0	6.0	2.50	2.00
O	BMWW-t	MT20	4.0	4.0		
P	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	1625	0	1625	0	0	5-8	2-6
J	1625	0	1625	0	0	5-8	2-6

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1156	720 / 0	0 / 0	0 / 0	0 / 0	435 / 0	0 / 0
J	1156	720 / 0	0 / 0	0 / 0	0 / 0	435 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.24 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC FROM	TO	MAX (PLF)	CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 52	-78.0	-78.0	0.15 (1)	10.00	
B-C	-1157 / 0	-78.0	-78.0	0.25 (1)	5.65	
C-D	-1508 / 0	-78.0	-78.0	0.82 (1)	4.25	
D-E	-1507 / 0	-78.0	-78.0	0.82 (1)	4.24	
E-F	-1507 / 0	-78.0	-78.0	0.82 (1)	4.24	
F-G	-1507 / 0	-78.0	-78.0	0.82 (1)	4.24	
G-H	-1157 / 0	-78.0	-78.0	0.25 (1)	5.65	
H-I	0 / 52	-78.0	-78.0	0.15 (1)	10.00	
P-B	-1606 / 0	0.0	0.0	0.23 (1)	6.52	
J-H	-1606 / 0	0.0	0.0	0.23 (1)	6.52	
P-O	0 / 0	-18.5	-18.5	0.17 (4)	10.00	
O-N	0 / 685	-18.5	-18.5	0.28 (4)	10.00	
N-M	0 / 1508	-18.5	-18.5	0.36 (1)	10.00	
M-L	0 / 1508	-18.5	-18.5	0.36 (1)	10.00	
L-K	0 / 686	-18.5	-18.5	0.29 (4)	10.00	
K-J	0 / 0	-18.5	-18.5	0.17 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O-C	-272 / 0	0.32 (1)
C-N	0 / 1142	0.18 (1)
N-D	-651 / 0	0.76 (1)
N-F	-1 / 0	0.00 (1)
L-F	-651 / 0	0.76 (1)
L-G	0 / 1144	0.18 (1)
K-G	-273 / 0	0.32 (1)
B-O	0 / 786	0.18 (1)
K-H	0 / 795	0.18 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.82/1.00 (F-G:1) , BC=0.36/1.00 (L-N:1) , WB=0.76/1.00 (F-L:1) , SSI=0.28/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1687	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (B) (INPUT = 0.93)
JSI METAL= 0.65 (M) (INPUT = 1.00)

A-18023177

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292576	H8	3	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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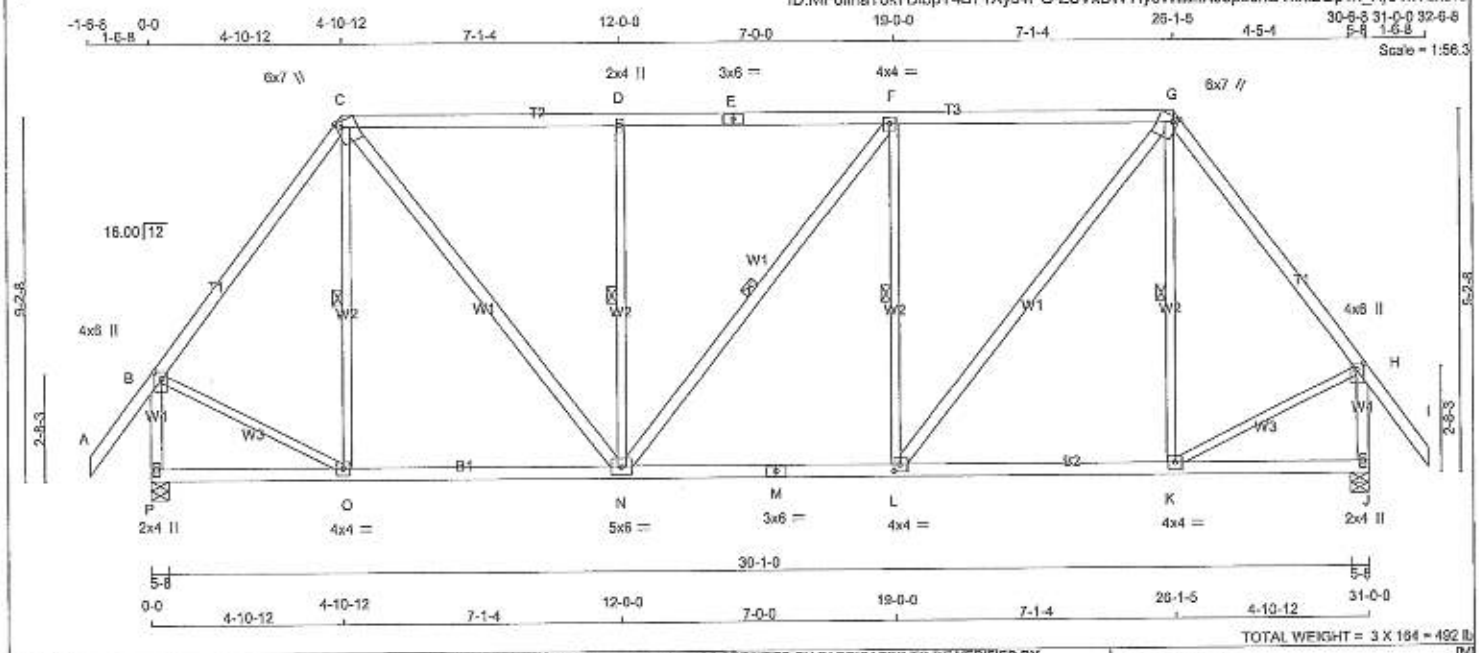

TOTAL WEIGHT = 3 X 164 = 492 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF
L - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW+p	MT20	4.0	6.0	2.00	2.25
C	TTWW+m	MT20	6.0	7.0	Edge	1.50
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTWW+m	MT20	6.0	7.0	Edge	1.50
H	TMW+p	MT20	4.0	6.0	2.00	2.25
J	BMV1+p	MT20	2.0	4.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMWW-t	MT20	4.0	4.0	2.00	1.75
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	5.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	1625	0	1625	0	0	5-8	2-6
J	1625	0	1625	0	0	5-8	2-6

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1156	720 / 0	0 / 0	0 / 0	0 / 0	435 / 0	0 / 0
J	1156	720 / 0	0 / 0	0 / 0	0 / 0	435 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.82 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-O, D-N, F-N, F-L, G-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 52	-78.0	-78.0	0.15 (1)	10.00	O- C	-200 / 10	0.11 (1)
B- C	-1192 / 0	-78.0	-78.0	0.42 (1)	5.41	C- N	0 / 929	0.15 (1)
C- D	-1284 / 0	-78.0	-78.0	0.63 (1)	4.83	N- D	-694 / 0	0.33 (1)
D- E	-1284 / 0	-78.0	-78.0	0.63 (1)	4.82	N- F	-2 / 0	0.00 (1)
E- F	-1284 / 0	-78.0	-78.0	0.63 (1)	4.82	L- F	-695 / 0	0.33 (1)
F- G	-1285 / 0	-78.0	-78.0	0.64 (1)	4.82	L- G	0 / 930	0.15 (1)
G- H	-1182 / 0	-78.0	-78.0	0.42 (1)	5.41	K- G	-201 / 10	0.11 (1)
H- I	0 / 52	-78.0	-78.0	0.15 (1)	10.00	B- O	0 / 776	0.17 (1)
P- B	-1592 / 0	0.0	0.0	0.23 (1)	6.55	K- H	0 / 776	0.17 (1)
J- H	-1592 / 0	0.0	0.0	0.23 (1)	6.55			
P- O	0 / 0	-16.5	-16.5	0.16 (4)	10.00			
O- N	0 / 703	-16.5	-16.5	0.25 (4)	10.00			
N- M	0 / 1285	-16.5	-16.5	0.31 (1)	10.00			
M- L	0 / 1285	-16.5	-16.5	0.31 (1)	10.00			
L- K	0 / 703	-16.5	-16.5	0.25 (4)	10.00			
K- J	0 / 0	-16.5	-16.5	0.16 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.84/1.00 (F-G:1) , BC=0.31/1.00 (L-N:1) , WB=0.33/1.00 (F-L:1) , SSI=0.26/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (P.I) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (B) (INPUT = 0.90)
JSI METAL= 0.47 (M) (INPUT = 1.00)

A-18023178

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292576	H9A	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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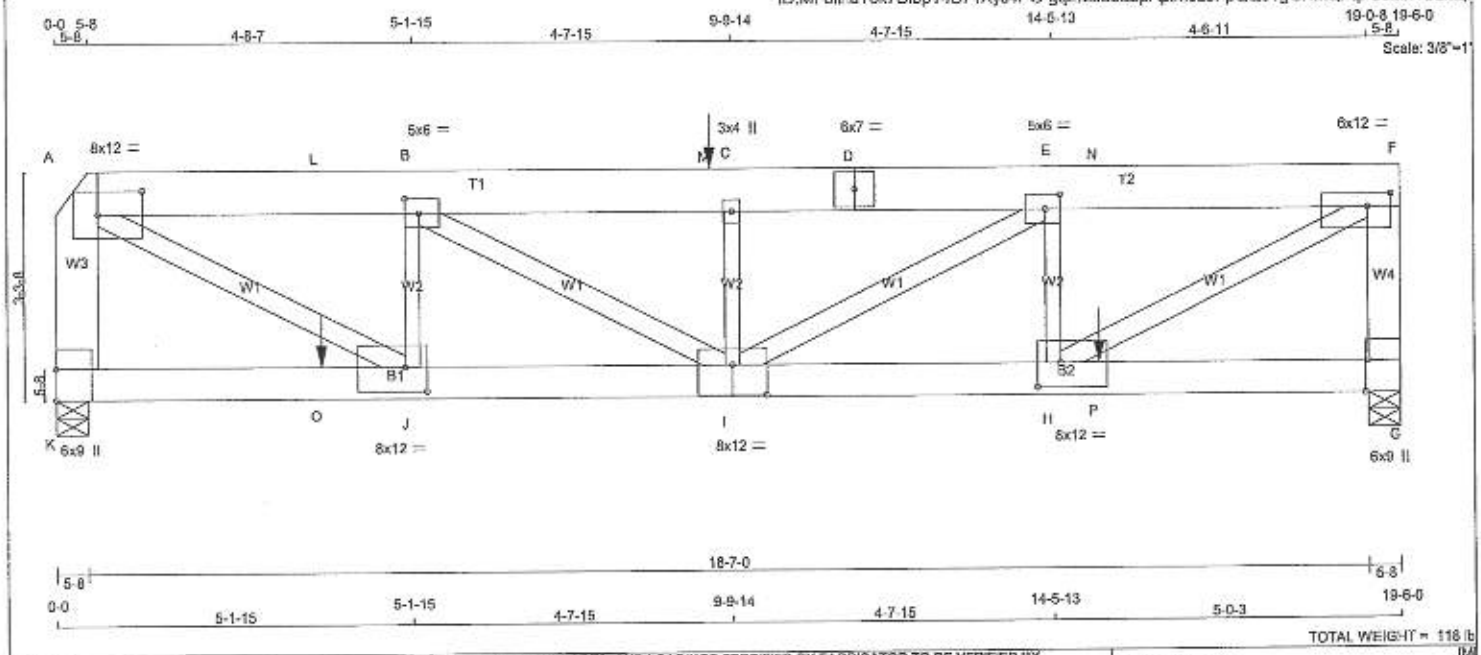

TOTAL WEIGHT = 118 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
G - F	2x6	DRY	No.2	SPF
K - A	2x6	DRY	No.2	SPF
K - I	2x6	DRY	2100F 1.8E	SPF
I - G	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x4	DRY	2100F 1.8E	SPF
H - E	2x3	DRY	No.2	SPF
J - B	2x3	DRY	No.2	SPF
I - C	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TVMW-I	MT20	8.0	12.0	4.00	7.75
B	TMWW-I	MT20	5.0	6.0	2.50	2.50
C	TMWI+w	MT20	3.0	4.0		
D	TS-I	MT20	6.0	7.0		
E	TMWW-I	MT20	5.0	6.0	2.50	2.50
F	TMWW-I	MT20	6.0	12.0	2.25	4.00
G	BMV1+I	MT20	6.0	9.0	Edge	0.50
H	BMWW-I	MT20	8.0	12.0	4.25	4.00
I	BSWWW-I	MT20	8.0	12.0	5.25	6.00
J	BMWW-I	MT20	8.0	12.0	4.25	3.75
K	BMV1+I	MT20	6.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES
1)

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

REGISTERED PROFESSIONAL ENGINEER
02/15/2018
EDWIN C. FOK
PROVINCE OF ONTARIO

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	4814	0	4814	0	0	5-8	5-8
K	4888	0	4888	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	3432	2094 / 0	0 / 0	0 / 0	0 / 0	1338 / 0	0 / 0
K	3485	2126 / 0	0 / 0	0 / 0	0 / 0	1359 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K
BEARING SIZE FACTOR = 1.15 AT JNT(S) G, K (BASED ON SUPPORT DEPTH = 1-8)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.70 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-L	-7339 / 0	-78.0	-78.0	0.36 (1)	3.45
L-B	-7339 / 0	-208.0	-208.0	0.36 (1)	3.45
B-M	-9373 / 0	-208.0	-208.0	0.67 (1)	2.70
M-C	-9373 / 0	-208.0	-208.0	0.67 (1)	2.70
C-D	-9373 / 0	-208.0	-208.0	0.60 (1)	2.80
D-E	-9373 / 0	-208.0	-208.0	0.60 (1)	2.80
E-N	-7152 / 0	-208.0	-208.0	0.35 (1)	3.50
N-F	-7152 / 0	-78.0	-78.0	0.35 (1)	3.50
G-F	-1148 / 0	0.0	0.0	0.40 (1)	5.23
K-A	-4138 / 0	0.0	0.0	0.39 (1)	5.92
K-O	0 / 0	-270.7	-270.7	0.42 (1)	10.00
O-J	0 / 0	-143.2	-143.2	0.42 (1)	10.00
J-I	0 / 7339	-143.2	-143.2	0.68 (1)	10.00
I-H	0 / 7152	-143.2	-143.2	0.62 (1)	10.00
H-P	0 / 0	-143.2	-143.2	0.33 (1)	10.00
P-G	0 / 0	-270.7	-270.7	0.33 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H-F	0 / 8163	0.67 (1)
A-J	0 / 8322	0.69 (1)
H-E	-2009 / 0	0.37 (1)
J-B	-2095 / 0	0.38 (1)
I-E	0 / 2580	0.21 (1)
B-I	0 / 2363	0.19 (1)
I-C	-2139 / 0	0.39 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
M	9-6-0	-1166	-1166	---	FRONT	VERT	TOTAL
O	3-10-4	-854	-854	---	FRONT	VERT	TOTAL
P	15-1-12	-854	-854	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 5-10-8
END DISTANCE = 19-6-0
END SPAN CARRIED = 5-10-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

GIRDER TYPE: CStdGirder
START DISTANCE = 15-1-12
START SPAN CARRIED = 8-7-0
END DISTANCE = 19-6-0
END SPAN CARRIED = 8-7-0
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 8-7-0
END DISTANCE = 3-10-4
END SPAN CARRIED = 8-7-0
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.85")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 660 (0.35")

A-18023179

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292576	H10T	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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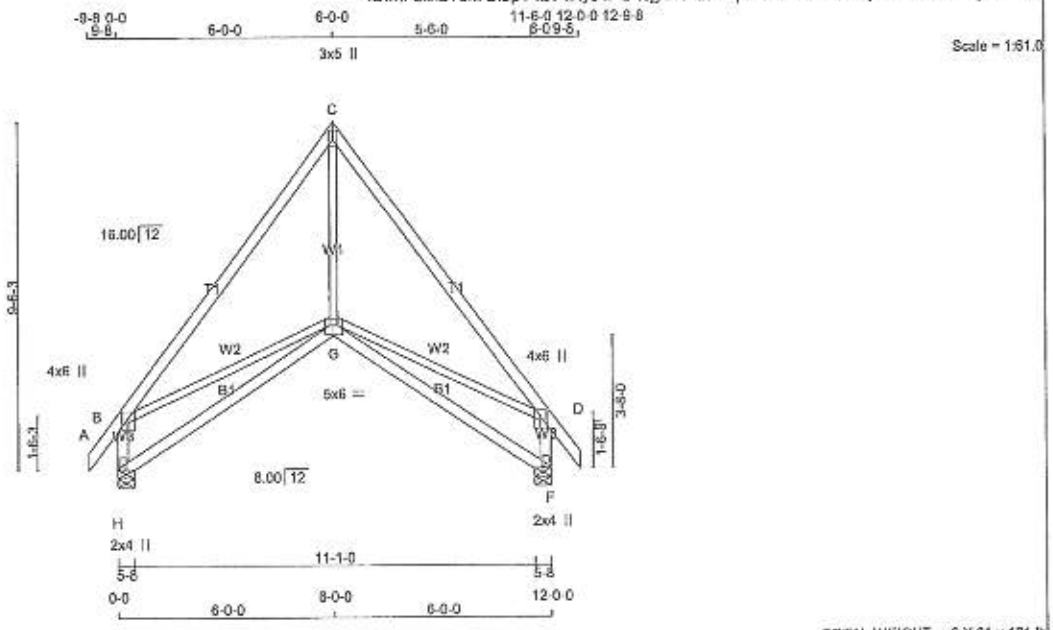

TOTAL WEIGHT = 2 X 61 = 121 lb

[M][F]

LUMBER

N. L, G, A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	2100F 1.8E	SPF
C - E	2x4	DRY	2100F 1.8E	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW+p	MT20	4.0	6.0	2.00	2.00
C	TTW+p	MT20	3.0	5.0	2.00	Edge
D	TMW+p	MT20	4.0	6.0	2.00	2.00
F	BMV1+p	MT20	2.0	4.0	Edge	
G	BBWWW-p	MT20	5.0	6.0	2.75	3.00
H	BMV1+p	MT20	2.0	4.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT SRG IN-SX	REQRD BRG IN-SX
H	650	0	650	0	0	5-8	1-8
F	650	0	650	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	462	290 / 0	0 / 0	0 / 0	0 / 0	172 / 0	0 / 0
F	462	290 / 0	0 / 0	0 / 0	0 / 0	172 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-78.0	-78.0	0.03 (1)		10.00
B- C	-533 / 0	-78.0	-78.0	0.24 (1)		6.25
C- D	-533 / 0	-78.0	-78.0	0.24 (1)		6.25
D- E	0 / 28	-78.0	-78.0	0.03 (1)		10.00
H- B	-594 / 0	0.0	0.0	0.07 (1)		7.81
F- D	-594 / 0	0.0	0.0	0.07 (1)		7.81
H- G	0 / 0	-18.5	-18.5	0.20 (4)		10.00
G- F	0 / 0	-18.5	-18.5	0.20 (4)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- C	0 / 385	0.09 (1)
B- G	0 / 348	0.08 (1)
G- D	0 / 348	0.08 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L., PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.40")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.40")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.24/1.00 (B-C:1) , BC=0.20/1.00 (G-H:4) , WB=0.09/1.00 (C-G:1) , SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (H) (INPUT = 0.90)
JSI METAL= 0.20 (F) (INPUT = 1.00)

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

A-18023180

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292576	H10TS	5	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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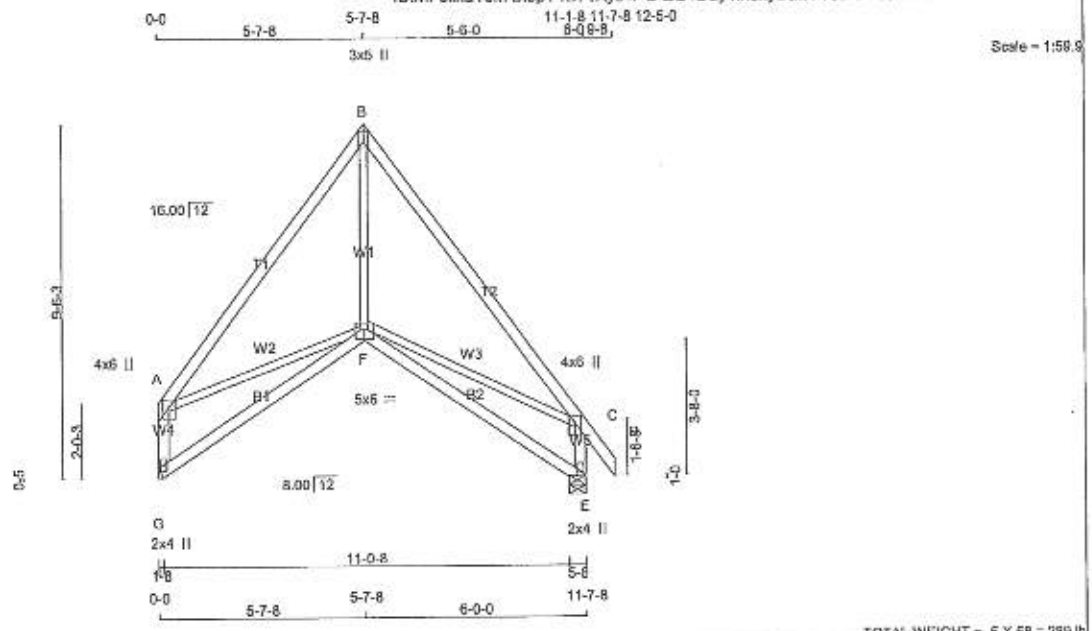

TOTAL WEIGHT = 5 X 58 = 289 lb

[MEF]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x6	DRY	2100F 1.8E	SPF
G - A	2x4	DRY	No.2	SPF
E - C	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.00	2.00
B	TTW+p	MT20	3.0	5.0	2.00	Edge
C	TMVW+p	MT20	4.0	6.0	2.00	2.00
E	BMV1+p	MT20	2.0	4.0	Edge	
F	BBW/WW-p	MT20	5.0	6.0	2.75	3.00
G	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	RECMD BRG IN-SX
G	561	0	561	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
E	632	0	632	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	400	244 / 0	0 / 0	0 / 0	0 / 0	156 / 0	0 / 0
E	449	282 / 0	0 / 0	0 / 0	0 / 0	167 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	-500 / 0	-78.0	-78.0	0.33 (1)	6.25
B-C	-500 / 0	-78.0	-78.0	0.24 (1)	6.25
C-D	0 / 28	-78.0	-78.0	0.03 (1)	10.00
G-A	-509 / 0	0.0	0.0	0.06 (1)	7.81
E-C	-578 / 0	0.0	0.0	0.05 (1)	7.81
G-F	0 / 0	-18.5	-18.5	0.17 (4)	10.00
F-E	0 / 0	-18.5	-18.5	0.20 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F-B	0 / 346	0.08 (1)
A-F	0 / 318	0.07 (1)
F-C	0 / 326	0.07 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.39")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.39")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.33/1.00 (A-B:1) , BC=0.20/1.00 (E-F:4) , WB=0.08/1.00 (B-F:1) , SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (E) (INPUT = 0.60)
JSI METAL= 0.28 (E) (INPUT = 1.00)

A-18023181

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292576	H11	5	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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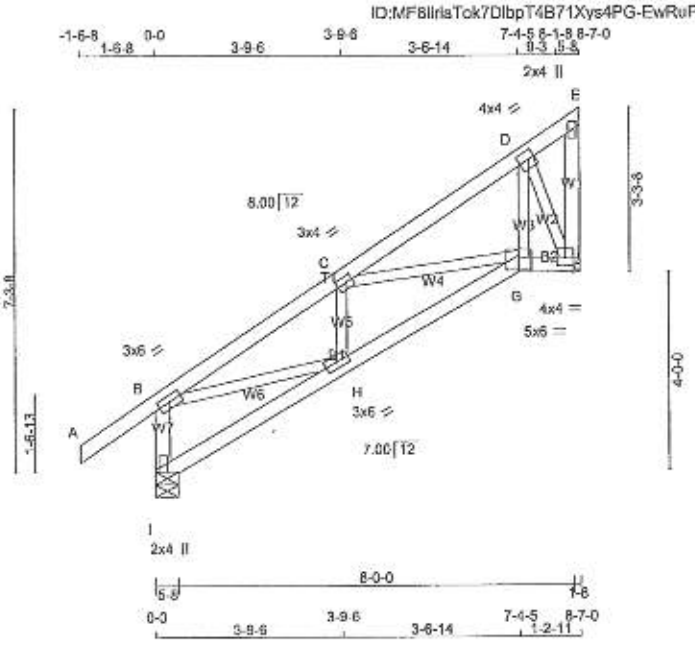

TOTAL WEIGHT = 5 X 40 = 201 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
I - B	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	3.0	6.0		
C	TMWW-t	MT20	3.0	4.0	1.50	1.50
D	TMWW-t	MT20	4.0	4.0	2.00	1.50
E	TMV+p	MT20	2.0	4.0		
F	BMVW1-t	MT20	4.0	4.0		
G	BBWW-t	MT20	5.0	6.0	3.00	3.00
H	BMWW-t	MT20	3.0	6.0		
I	BMV1+p	MT20	2.0	4.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	980	0	980	0	0	5-8	1-8
F	854	0	854	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	696	440 / 0	0 / 0	0 / 0	0 / 0	257 / 0	0 / 0
F	609	372 / 0	0 / 0	0 / 0	0 / 0	237 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.12 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT, LOAD LC (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
I-B	-917 / 0	0.0	0.0	0.11 (1)	7.81
A-B	0 / 35	-78.0	-78.0	0.16 (1)	10.00
B-C	-1321 / 0	-160.8	-160.8	0.37 (1)	5.12
C-D	-571 / 0	-160.8	-160.8	0.34 (1)	6.25
D-E	-68 / 0	-160.8	-160.8	0.19 (1)	6.25
F-E	0 / 15	0.0	0.0	0.00 (1)	10.00
I-H	0 / 8	-38.2	-38.2	0.14 (4)	10.00
H-G	0 / 1297	-38.2	-38.2	0.30 (1)	10.00
G-F	0 / 429	-38.2	-38.2	0.09 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B-H	0 / 1145	0.28 (1)
H-C	-266 / 15	0.05 (1)
C-G	-996 / 0	0.18 (1)
G-D	0 / 695	0.17 (1)
D-F	-952 / 0	0.17 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	21.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	34.4	PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: GPrimeHip
SIDE SETBACK = 0-0
END SETBACK = 6-3-0
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSI.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.29")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.26")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.37/1.00 (B-C:1) , BC=0.30/1.00 (G-H:1) , WB=0.28/1.00 (B-H:1) , SSI=0.25/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (D) (INPUT = 0.90)
JSI METAL= 0.43 (I) (INPUT = 1.00)

A-18023182

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292576	H12A	1	2	TRUSS DESC.	

Alps Roof Truss, Maple

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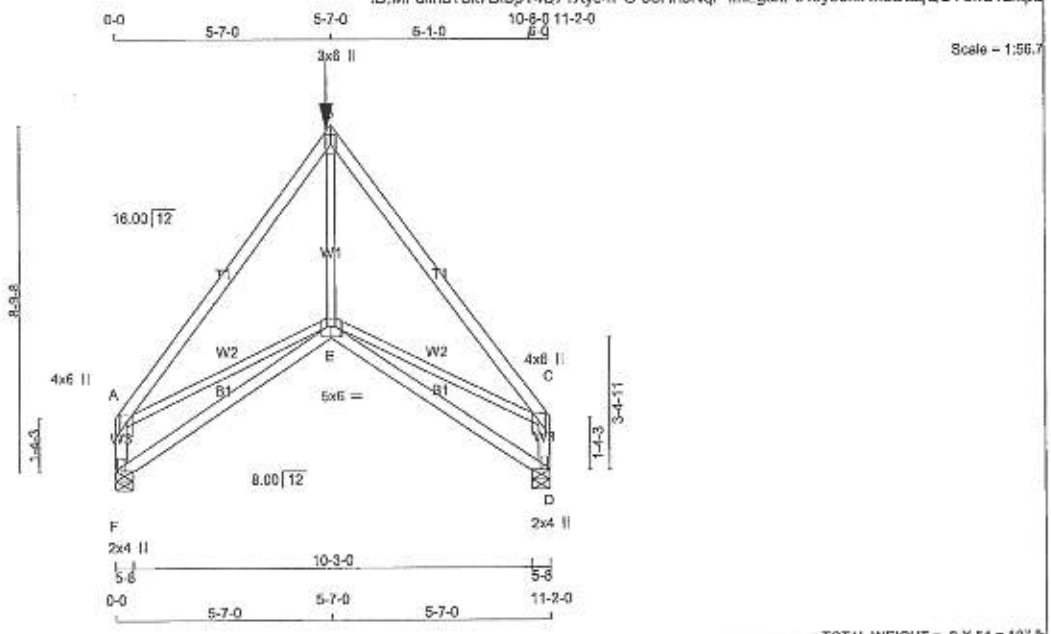


TOTAL WEIGHT = 2 X 54 = 107 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- B	1	12	SIDE(61.0)
B- C	1	12	SIDE(61.0)
F- A	1	12	TOP
D- C	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- E	1	12	SIDE(14.6)
E- D	1	12	SIDE(14.6)
WEBS : (0.122"X3") SPIRAL NAILS			
E- B	1	6	SIDE(179.6)
2x3	1	6	

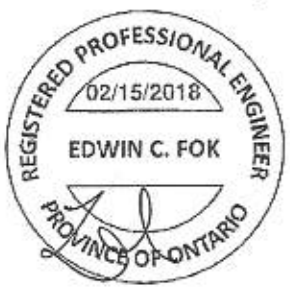
NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1292	0	1292	0	0	5-8	1-8
D	1292	0	1292	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	936	486 / 0	0 / 0	0 / 0	0 / 0	450 / 0	0 / 0
D	936	486 / 0	0 / 0	0 / 0	0 / 0	450 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.21 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-1745 / 0	-78.0	-78.0	0.31 (1)	6.21		E- B	0 / 1200	0.15 (1)
B- C	-1745 / 0	-78.0	-78.0	0.31 (1)	6.21		A- E	0 / 1144	0.14 (1)
F- A	-1153 / 0	0.0	0.0	0.07 (1)	7.81		E- C	0 / 1144	0.14 (1)
D- C	-1153 / 0	0.0	0.0	0.07 (1)	7.81				
F- E	0 / 0	-49.7	-49.7	0.25 (4)	10.00				
E- D	0 / 0	-49.7	-49.7	0.25 (4)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
B	5-7-0	-1157	-1157	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.37")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.37")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.31/1.00 (A-B:1) , BC=0.25/1.00 (E-F:4) , WB=0.15/1.00 (B-E:1) , SSI=0.09/1.00 (E-F:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (D) (INPUT = 0.90)
JSI METAL= 0.30 (C) (INPUT = 1.00)

A-18023183

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292576	H12A	1	2	TRUSS DESC.	

Alpa Roof Truss, Maple

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.00	2.00
B	TTW+p	MT20	3.0	6.0		
C	TMVW+p	MT20	4.0	6.0	2.00	2.00
D	BMV1+p	MT20	2.0	4.0	Edge	
E	BBWWW+p	MT20	5.0	6.0	2.75	3.00
F	BMV1+p	MT20	2.0	4.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1157.1 lbs FACTORED DOWN AT 5-7-0 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

REGISTERED PROFESSIONAL ENGINEER
02/15/2018
EDWIN C. FOK
PROVINCE OF ONTARIO

A-180231826

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292576	H12T	2	1	TRUSS DESC.	

Ajax Roof Truss, Maple

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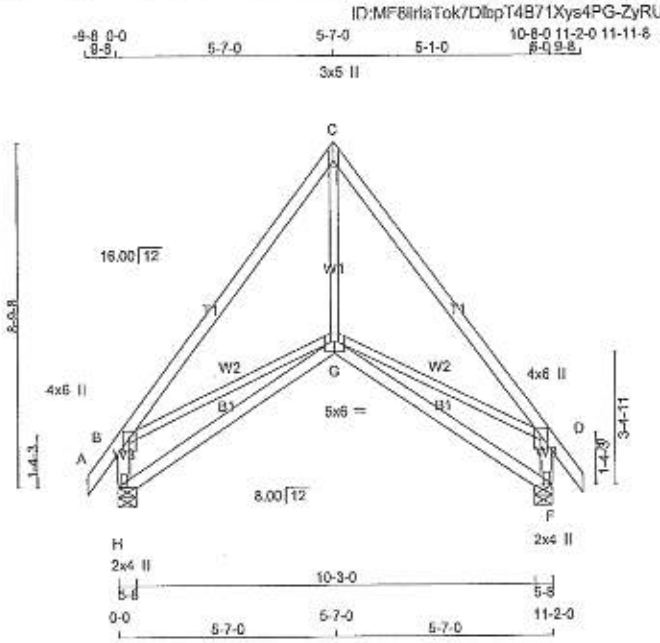

TOTAL WEIGHT = 2 X 57 = 113 lb (MIF)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	8.0	2.00	2.00
C	TTW+p	MT20	3.0	5.0	2.00	Edge
D	TMVW+p	MT20	4.0	8.0	2.00	2.00
F	BMV1+p	MT20	2.0	4.0	Edge	
G	BBWWW-p	MT20	5.0	6.0	2.75	3.00
H	BMV1+p	MT20	2.0	4.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	609	0	609	0	0	5-8	1-8
F	609	0	609	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	433	273 / 0	0 / 0	0 / 0	0 / 0	161 / 0	0 / 0
F	433	273 / 0	0 / 0	0 / 0	0 / 0	161 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 28	-78.0	-78.0	0.05 (1)	10.00	G- C	0 / 369	0.08 (1)
B- C	-503 / 0	-78.0	-78.0	0.32 (1)	6.25	B- G	0 / 329	0.07 (1)
C- D	-503 / 0	-78.0	-78.0	0.32 (1)	6.25	G- D	0 / 330	0.07 (1)
D- E	0 / 28	-78.0	-78.0	0.05 (1)	10.00			
H- B	-556 / 0	0.0	0.0	0.08 (1)	7.81			
F- D	-556 / 0	0.0	0.0	0.08 (1)	7.81			
H- G	0 / 0	-18.5	-18.5	0.17 (4)	10.00			
G- F	0 / 0	-18.5	-18.5	0.17 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.37")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.37")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.32/1.00 (B-C:1) , BC=0.17/1.00 (G-H:4) , WB=0.08/1.00 (C-G:1) , SSI=0.10/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.71 (F) (INPUT = 0.90)
JSI METAL= 0.27 (F) (INPUT = 1.00)

A-18023184

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292576	L1	2	1	TRUSS DESC.	

Wpa Roof Truss, Maple

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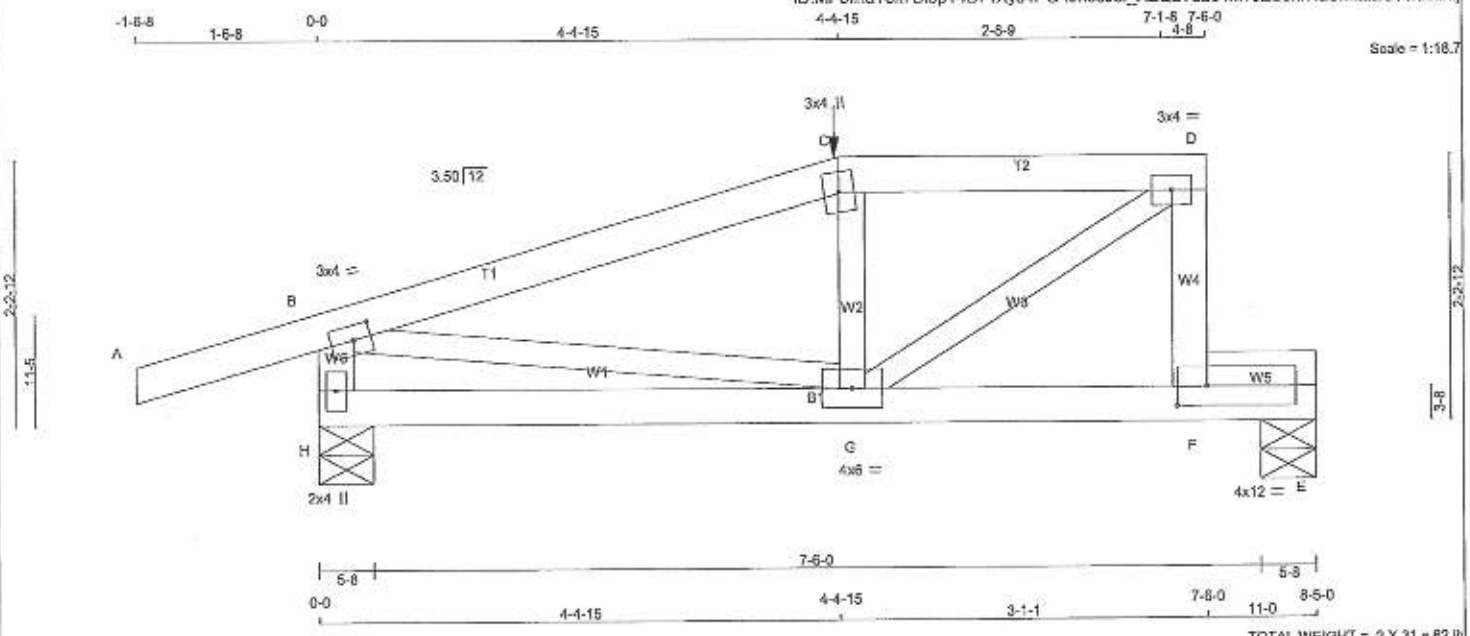


TOTAL WEIGHT = 2 X 31 = 62 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

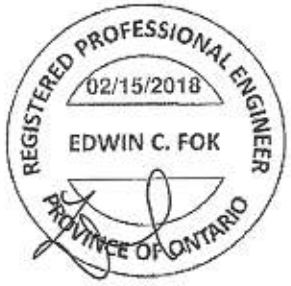
DRY: SEASONED LUMBER.

PLATES (Table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	3.0	4.0	1.50	1.75
C	TTW+m	MT20	3.0	4.0		
D	TMVW-t	MT20	3.0	4.0		
F	BMVW-t	MT20	4.0	12.0	2.00	3.00
G	BMWWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	2.0	4.0		

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 28.7 lbs FACTORED DOWN AT 4-4-15 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	539	0	539	0	0	5-8	1-8
E	353	0	353	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	392	248 / 0	0 / 0	0 / 0	0 / 0	133 / 0	0 / 0
E	253	148 / 0	0 / 0	0 / 0	0 / 0	105 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (1)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 17	-78.0	-78.0	0.15	(1)	10.00	G- C	-174 / 0	0.03 (1)
B- C	-528 / 0	-78.0	-78.0	0.29	(1)	6.25	G- D	0 / 588	0.15 (1)
C- D	-488 / 0	-78.0	-78.0	0.14	(1)	6.25	D- G	0 / 511	0.13 (1)
F- D	-446 / 0	0.0	0.0	0.06	(1)	7.81			
H- B	-514 / 0	0.0	0.0	0.06	(1)	7.81			
H- G	0 / 0	-18.5	-18.5	0.14	(4)	10.00			
G- F	0 / 0	-18.5	-18.5	0.47	(1)	10.00			
F- E	0 / 0	-18.5	-18.5	0.20	(1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE
C	4-4-15	-29	-29	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 4-4-15
RIGHT SETBACK = 0-0
END SETBACK = 2-0-0
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDTL LOADS BASED ON 65 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.28")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.28")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.29/1.00 (B-C:1) , BC=0.47/1.00 (F-G:1) , WB=0.15/1.00 (D-G:1) , SSI=0.15/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1658

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (D) (INPUT = 0.90)
JSI METAL= 0.23 (B) (INPUT = 1.00)

A-18023185

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292576	L2	4	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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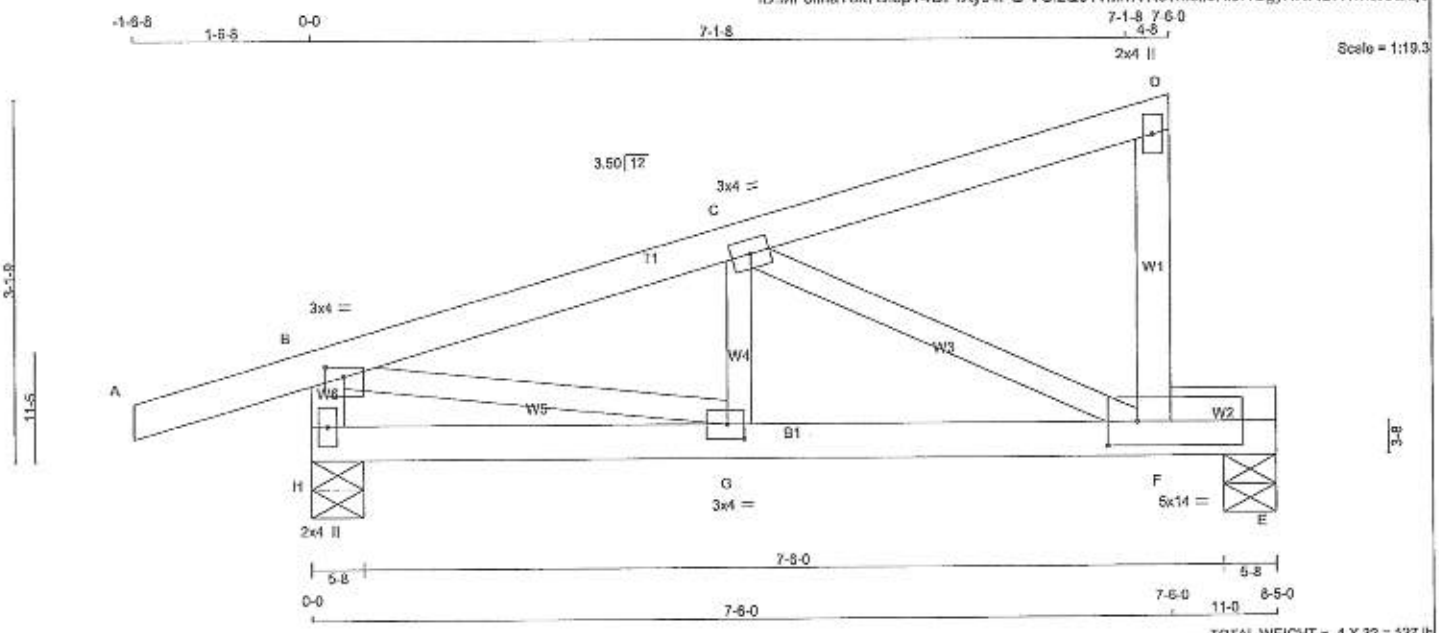

TOTAL WEIGHT = 4 X 32 = 127 lb (M)(F)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	3.0	4.0	1.00	2.00
C	TMVW-t	MT20	3.0	4.0		
D	TMV+p	MT20	2.0	4.0		
F	BMVWW-t	MT20	5.0	14.0	2.50	3.00
G	BMVW-t	MT20	3.0	4.0	1.50	1.75
H	BMV1+p	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H	525	0	525	0	0	5-8	1-8
E	338	0	338	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	372	241 / 0	0 / 0	0 / 0	0 / 0	131 / 0	0 / 0
E	243	148 / 0	0 / 0	0 / 0	0 / 0	102 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 17	-78.0	-78.0	1	0.14 (1)	10.00
B-C	-612 / 0	-78.0	-78.0	1	0.13 (1)	6.25
C-D	-9 / 0	-78.0	-78.0	1	0.12 (1)	10.00
F-D	-115 / 0	0.0	0.0	1	0.02 (1)	7.81
H-B	-514 / 0	0.0	0.0	1	0.05 (1)	7.81
H-G	0 / 0	-18.5	-18.5	4	0.12 (4)	10.00
G-F	0 / 596	-18.5	-18.5	1	0.51 (1)	10.00
F-E	0 / 0	-18.5	-18.5	1	0.17 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G-C	0 / 102	0.04 (4)
B-G	0 / 605	0.14 (1)
C-F	-357 / 0	0.18 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.28")
CALCULATED VERT. DEFL.(LL) = L/ 869 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.28")
CALCULATED VERT. DEFL.(TL) = L/ 912 (0.11")

CSI: TC=0.14/1.00 (A-B:1) , BC=0.51/1.00 (F-G:1) , WB=0.18/1.00 (C-F:1) , SBI=0.13/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 I.S BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	619	364	1667	788	1997	1556

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (G) (INPUT = 0.80)
JSI METAL= 0.23 (B) (INPUT = 1.00)

A-18023186

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292576	L2A	4	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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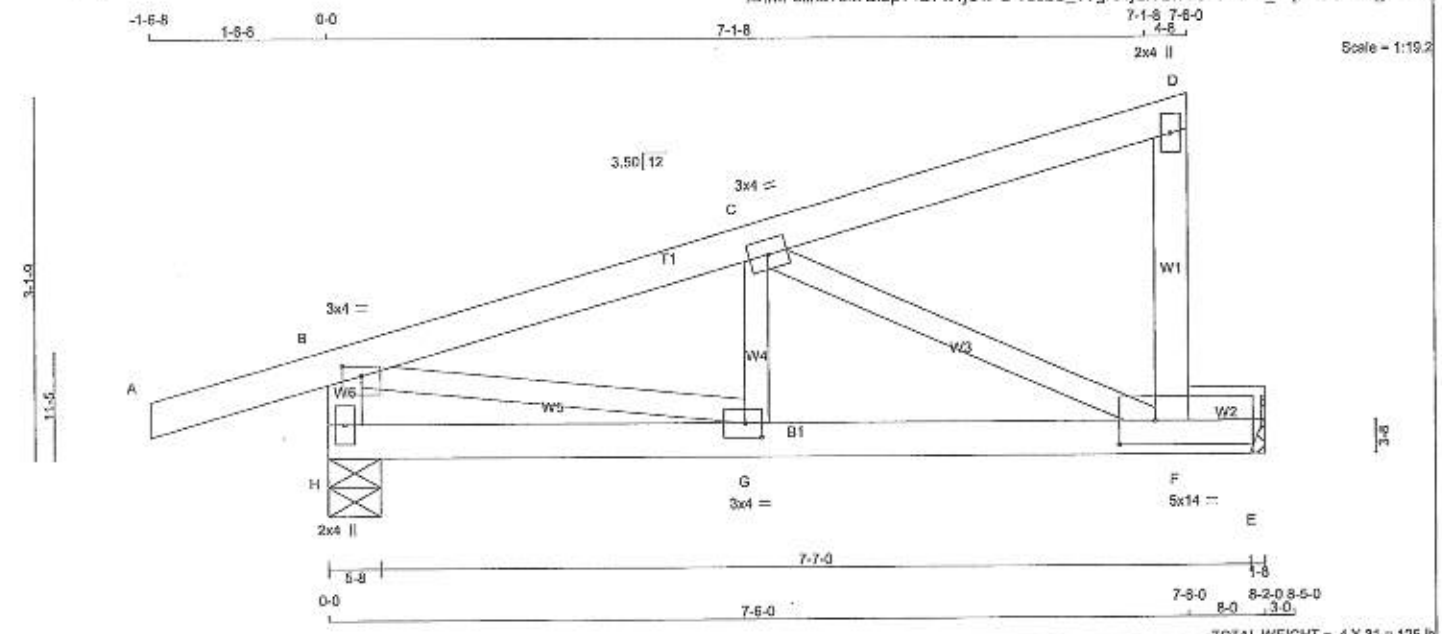

TOTAL WEIGHT = 4 X 31 = 125 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	3.0	4.0	1.00	2.00
C	TMWW-t	MT20	3.0	4.0		
D	TMV+p	MT20	2.0	4.0		
F	BMWWW-t	MT20	5.0	14.0	2.50	3.75
G	BMWW-t	MT20	3.0	4.0	1.50	1.75
H	BMV1+p	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H	515	0	515	0	0	5-8	1-8
E	344	0	344	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	365	237 / 0	0 / 0	0 / 0	0 / 0	128 / 0	0 / 0
E	246	145 / 0	0 / 0	0 / 0	0 / 0	102 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 17	-78.0	-78.0	0.14 (1)	10.00	
B- C	-575 / 0	-78.0	-78.0	0.13 (1)	6.25	
C- D	-9 / 0	-78.0	-78.0	0.13 (1)	10.00	
F- D	-114 / 0	0.0	0.0	0.02 (1)	7.81	
H- B	-499 / 0	0.0	0.0	0.05 (1)	7.81	
H- G	0 / 0	-18.5	-18.5	0.11 (4)	10.00	
G- F	0 / 564	-18.5	-18.5	0.40 (1)	10.00	
F- E	0 / 0	-18.5	-18.5	0.13 (1)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- C	0 / 91	0.03 (4)
B- G	0 / 572	0.13 (1)
C- F	-621 / 0	0.17 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.27")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.27")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.14/1.00 (A-B:1) , BC=0.40/1.00 (F-G:1) , WB=0.17/1.00 (C-F:1) , SSI=0.13/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1667	788	1987	1056

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (G) (INPUT = 0.90)
JSI METAL= 0.21 (S) (INPUT = 1.00)

A-18023187

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292577	H1T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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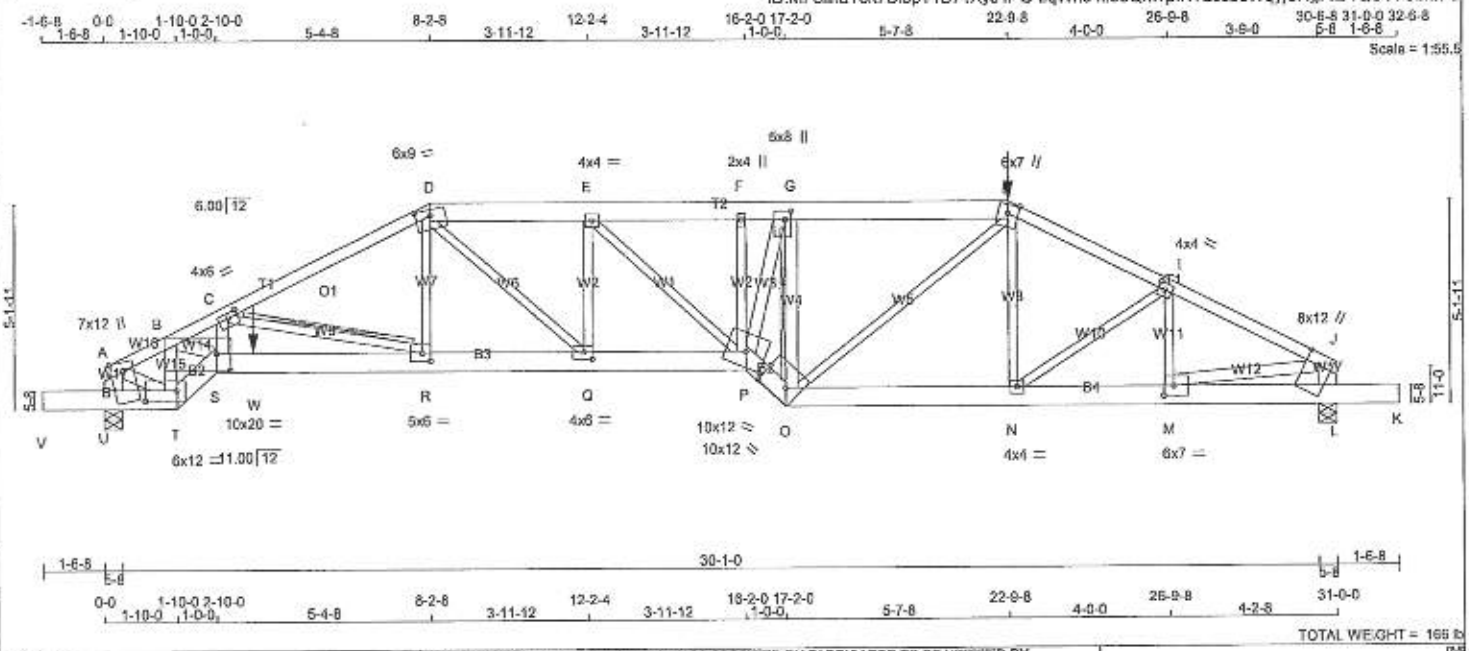

TOTAL WEIGHT = 166 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	1650F 1.5E	SPF
D - H	2x8	DRY	No.2	SPF
H - J	2x4	DRY	1650F 1.5E	SPF
U - A	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
V - T	2x6	DRY	No.2	SPF
T - S	2x6	DRY	No.2	SPF
S - P	2x6	DRY	1650F 1.5E	SPF
P - O	2x6	DRY	No.2	SPF
O - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
B - S	2x4	DRY	No.2	SPF
S - C	2x4	DRY	No.2	SPF
P - G	2x4	DRY	No.2	SPF
O - G	2x4	DRY	No.2	SPF
A - T	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A, B, J, L, S, U						
A	TMBMVW1*+n	MT20	7.0	12.0	Edge	
C	TMWW-t	MT20	4.0	6.0	1.50	3.00
D	TTWW-m	MT20	6.0	9.0	2.00	4.50
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TMWW-t	MT20	5.0	8.0	2.75	1.75
H	TTWW+m	MT20	6.0	7.0	Edge	
I	TMWW-t	MT20	4.0	4.0	2.00	1.75
J	TMBMVW1*+h	MT20	8.0	12.0	Edge	
M	BMWW-t	MT20	6.0	7.0	3.00	2.75
N	BMWW-t	MT20	4.0	4.0		
O	BBWW-h	MT20	10.0	12.0	3.75	8.00
P	BBWWW-m	MT20	10.0	12.0	4.50	6.00
Q	BMWW-t	MT20	4.0	6.0	2.00	2.50

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	3168	0	3168	0	0	5-8	4-9
L	2926	0	2926	0	0	5-8	3-12

ALLOW FOR 0.3" OF HORIZONTAL MOVEMENT DUE TO TOTAL LOAD

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	2251	1416 / 0	0 / 0	0 / 0	0 / 0	835 / 0	0 / 0
L	2089	1258 / 0	0 / 0	0 / 0	0 / 0	831 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.45 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT C-R, G-O

FASTEN T AND L-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-3809 / 0	-78.0	-78.0	0.23 (1)	3.90
B- C	-6139 / 0	-78.0	-78.0	0.46 (1)	2.45
C- D	-5822 / 0	-78.0	-78.0	0.67 (1)	3.00
D- E	-6086 / 0	-78.0	-78.0	0.30 (1)	3.34
E- F	-6337 / 0	-78.0	-78.0	0.46 (1)	3.00
F- G	-6347 / 0	-153.5	-153.5	0.50 (1)	3.00
G- H	-5195 / 0	-153.5	-153.5	0.49 (1)	3.38
H- I	-4515 / 0	-78.0	-78.0	0.30 (1)	3.59
I- J	-4333 / 0	-78.0	-78.0	0.30 (1)	3.67
U- A	-2940 / 0	0.0	0.0	0.21 (1)	6.09
L- J	-2574 / 0	0.0	0.0	0.19 (1)	6.33
V- U	0 / 0	-96.5	-96.5	0.08 (1)	10.00
U- T	-1 / 0	-18.5	-18.5	0.08 (1)	10.00
T- S	0 / 4050	-18.5	-18.5	0.53 (1)	10.00
S- W	0 / 7188	-18.5	-18.5	0.82 (1)	10.00
W- R	0 / 7188	-112.0	-112.0	0.82 (1)	10.00
R- Q	0 / 5003	-112.0	-112.0	0.58 (1)	10.00
Q- P	0 / 6085	-112.0	-112.0	0.69 (1)	10.00
P- O	0 / 6815	-36.4	-36.4	0.69 (1)	10.00
O- N	0 / 4037	-36.4	-36.4	0.69 (1)	10.00
N- M	0 / 3887	-36.4	-36.4	0.59 (1)	10.00
M- L	0 / 0	-36.4	-36.4	0.11 (1)	10.00
L- K	0 / 0	-96.5	-96.5	0.08 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- B	-3904 / 0	0.44 (1)
B- S	0 / 4802	0.85 (1)
S- C	0 / 1278	0.23 (1)
C- R	-2210 / 0	0.54 (1)
R- D	0 / 1181	0.29 (1)
D- Q	0 / 1496	0.37 (1)
Q- E	-706 / 0	0.18 (1)
E- P	0 / 345	0.09 (1)
P- F	0 / 362	0.09 (1)
P- G	0 / 4489	0.80 (1)
O- G	-5436 / 0	0.86 (1)
O- H	0 / 1505	0.37 (1)
N- H	0 / 217	0.08 (4)
N- I	0 / 187	0.05 (1)
M- I	-530 / 0	0.10 (1)
A- T	0 / 3555	0.63 (1)
M- J	0 / 3932	0.70 (1)

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	21.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CSldGirder
START DISTANCE = 3-9-4
START SPAN CARRIED = 5-10-8
END DISTANCE = 18-2-0
END SPAN CARRIED = 5-10-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 6-2-8
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 14-10-0 OF SPAN MEASURED FROM THE RIGHT.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 9.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.31")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 619 (0.50")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 207 (0.09")

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CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292577	H1T	1	1	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
R	BMWW-I	MT20	5.0	6.0	2.50	2.50
S	TMBBWW*-I	MT20	10.0	20.0	5.00	4.25
T	BBWW-I	MT20	6.0	12.0	3.00	8.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 466.7 lbs FACTORED DOWN AT 22-9-8 ON TOP CHORD, AND 407.4 lbs FACTORED DOWN AT 3-9-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
H	22-9-8	-467	-467	—	FRONT	VERT	TOTAL
W	3-9-4	-407	-407	—	FRONT	VERT	TOTAL

ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 120 (0.17")

CSI: TC=0.67/1.00 (C-D:1) , BC=0.88/1.00 (D-P:1) , WB=0.86/1.00 (C-O:1) , SSI=0.41/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1997	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (M) (INPUT = 0.90)
JSI METAL= 0.97 (A) (INPUT = 1.00)

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

A-180231886-1

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292577	H2T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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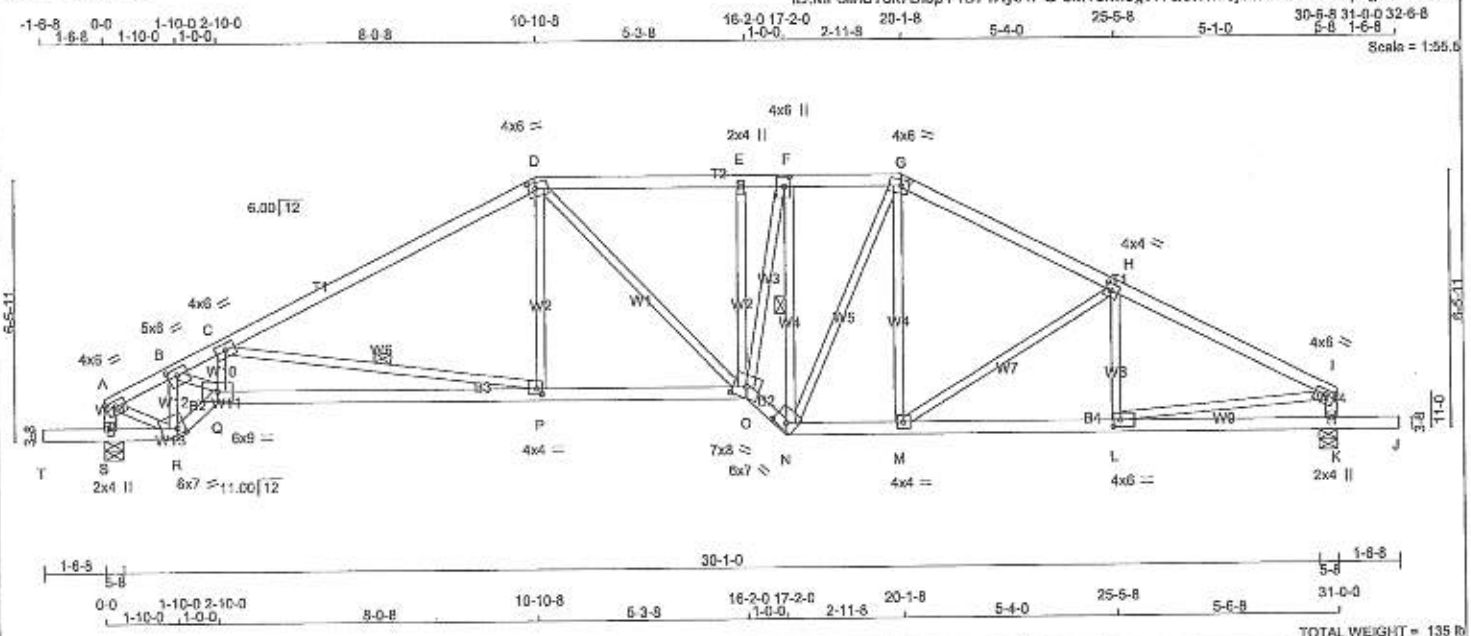

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
S - A	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
R - Q	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMW-I	MT20	4.0	6.0	1.75	Edge
B	TMWW-I	MT20	5.0	6.0	2.00	2.75
C	TMWW-I	MT20	4.0	6.0		
D	TTWW-m	MT20	4.0	6.0	1.75	2.25
E	TMW+w	MT20	2.0	4.0		
F	TMWW+I	MT20	4.0	6.0	3.00	1.50
G	TTWW-m	MT20	4.0	6.0	1.75	2.25
H	TMWW-I	MT20	4.0	4.0	2.00	1.75
I	TMWW-I	MT20	4.0	6.0	1.50	2.75
K	BMV1+p	MT20	2.0	4.0		
L	BMWW-I	MT20	4.0	6.0	2.00	2.00
M	BMWW-I	MT20	4.0	4.0		
N	BBWW-h	MT20	6.0	7.0	2.00	4.00
O	BBWWW-m	MT20	7.0	8.0	3.25	4.00
P	BMWW-I	MT20	4.0	4.0	1.50	1.50
Q	BBWW-I	MT20	6.0	8.0		
R	BBWW-m	MT20	6.0	7.0	1.75	2.25
S	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

REGISTERED PROFESSIONAL ENGINEER
02/15/2018
EDWIN C. FOK
PROVINCE OF ONTARIO

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	1648	0	1648	0	0	5-8	2-7
K	1640	0	1640	0	0	5-8	2-7

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1175	717 / 0	0 / 0	0 / 0	0 / 0	458 / 0	0 / 0
K	1169	714 / 0	0 / 0	0 / 0	0 / 0	456 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.01 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-P, F-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	-1651 / 0	-78.0	-78.0	0.18 (1)	5.00	
B-C	-3640 / 0	-78.0	-78.0	0.61 (1)	3.01	
C-D	-2273 / 0	-78.0	-78.0	0.81 (1)	3.69	
D-E	-2230 / 0	-78.0	-78.0	0.33 (1)	4.28	
E-F	-2223 / 0	-78.0	-78.0	0.21 (1)	4.40	
F-G	-1861 / 0	-78.0	-78.0	0.11 (1)	4.84	
G-H	-1929 / 0	-78.0	-78.0	0.34 (1)	4.53	
H-I	-2243 / 0	-78.0	-78.0	0.37 (1)	4.23	
S-A	-1419 / 0	0.0	0.0	0.14 (1)	6.84	
K-I	-1447 / 0	0.0	0.0	0.15 (1)	6.79	
T-S	0 / 0	-98.5	-98.5	0.16 (1)	10.00	
S-R	0 / 0	-18.5	-18.5	0.12 (1)	10.00	
R-Q	0 / 1799	-18.5	-18.5	0.29 (1)	10.00	
Q-P	0 / 3444	-18.5	-18.5	0.71 (1)	10.00	
P-O	0 / 2018	-18.5	-18.5	0.48 (1)	10.00	
O-N	0 / 2480	-18.5	-18.5	0.40 (1)	10.00	
N-M	0 / 1709	-18.5	-18.5	0.32 (1)	10.00	
M-L	0 / 2025	-18.5	-18.5	0.37 (1)	10.00	
L-K	0 / 0	-18.5	-18.5	0.12 (4)	10.00	
K-J	0 / 0	-98.5	-98.5	0.18 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R-B	-1810 / 0	0.26 (1)
B-Q	0 / 2462	0.55 (1)
Q-C	0 / 123	0.04 (4)
C-P	-1453 / 0	0.67 (1)
P-D	0 / 366	0.08 (4)
D-O	0 / 301	0.07 (1)
O-E	-384 / 0	0.17 (1)
O-F	0 / 1915	0.43 (1)
N-F	-1953 / 0	0.53 (1)
N-G	0 / 350	0.08 (1)
M-G	0 / 279	0.06 (1)
M-H	-381 / 0	0.29 (1)
L-H	-168 / 25	0.04 (1)
A-R	0 / 1649	0.35 (1)
L-I	0 / 2042	0.46 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.37")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 404 (0.05")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 208 (0.09")

CSI: TC=0.81/1.00 (C-D:1) , BC=0.71/1.00 (P-Q:1) , WB=0.67/1.00 (C-P:1) , SSI=0.37/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	789	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.59 (Q) (INPUT = 1.00)

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CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292577	H3T	1	1	TRUSS DESC.	

Npe Roof Truss, Maple

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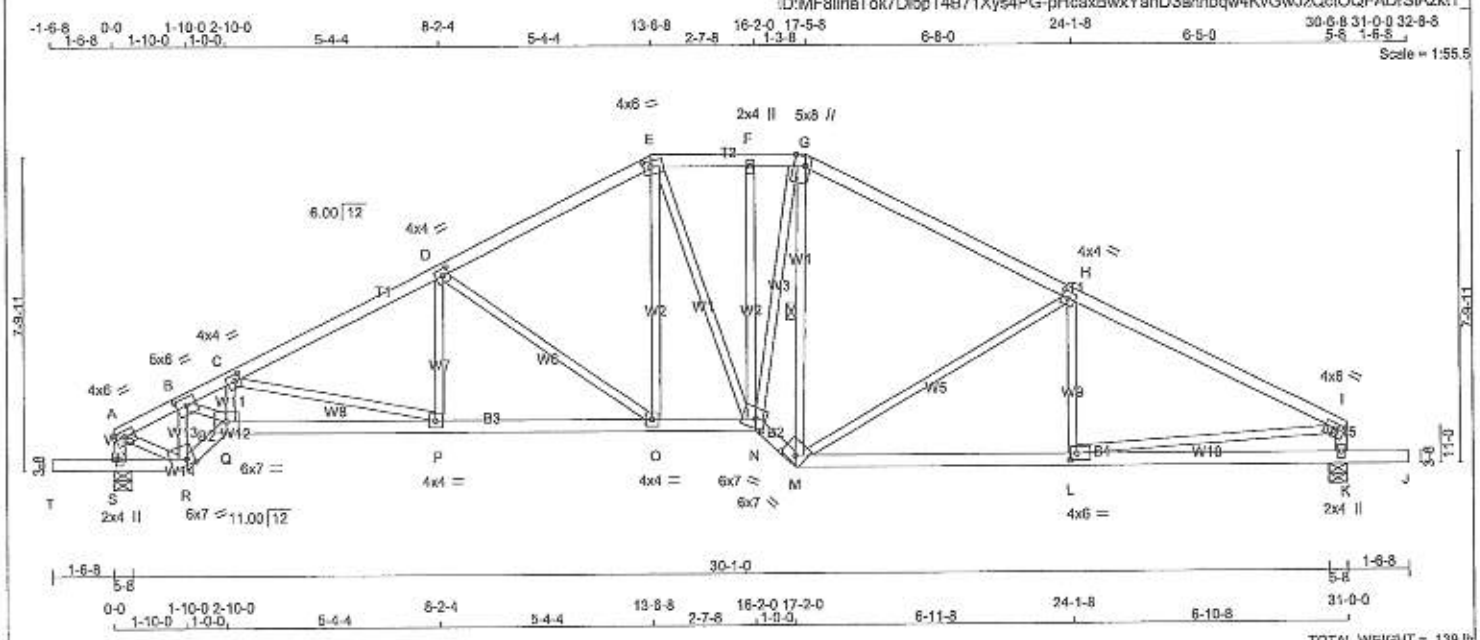

TOTAL WEIGHT = 139 lb

MiTek

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
S - A	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
R - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-t	MT20	4.0	6.0	1.75	Edge
B	TMWW-t	MT20	5.0	6.0	2.25	3.00
C, D, H						
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
E	TTWW-m	MT20	4.0	8.0	1.75	2.25
F	TMW+w	MT20	2.0	4.0		
G	TTWW+m	MT20	5.0	8.0	Edge	3.50
I	TMWW-t	MT20	4.0	8.0	1.50	2.75
K	BMV1+p	MT20	2.0	4.0		
L	BMWW-t	MT20	4.0	6.0	2.00	2.00
M	BBWW-h	MT20	6.0	7.0	2.00	4.00
N	BBWWW-m	MT20	6.0	7.0	2.75	3.00
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	4.0	4.0		
Q	BBWW-t	MT20	6.0	7.0		
R	BBWW-m	MT20	6.0	7.0	1.75	2.25
S	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	1648	0	1648	0	0	5-8	2-7
K	1640	0	1640	0	0	5-8	2-7

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1175	717 / 0	0 / 0	0 / 0	0 / 0	458 / 0	0 / 0
K	1169	714 / 0	0 / 0	0 / 0	0 / 0	456 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.65 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	-1687 / 0	-78.0	-78.0	0.11 (1)		5.03
B-C	-3481 / 0	-78.0	-78.0	0.18 (1)		3.65
C-D	-2821 / 0	-78.0	-78.0	0.37 (1)		3.97
D-E	-1941 / 0	-78.0	-78.0	0.33 (1)		4.53
E-F	-1750 / 0	-78.0	-78.0	0.12 (1)		4.95
F-G	-1748 / 0	-78.0	-78.0	0.07 (1)		5.03
G-H	-1710 / 0	-78.0	-78.0	0.53 (1)		4.50
H-I	-2251 / 0	-78.0	-78.0	0.61 (1)		3.95
S-A	-1419 / 0	0.0	0.0	0.14 (1)		6.84
K-I	-1441 / 0	0.0	0.0	0.14 (1)		6.81
T-S	0 / 0	-96.5	-96.5	0.16 (1)		10.00
S-R	0 / 0	-18.5	-18.5	0.12 (1)		10.00
R-Q	0 / 1861	-18.5	-18.5	0.30 (1)		10.00
Q-P	0 / 3126	-18.5	-18.5	0.57 (1)		10.00
P-O	0 / 2359	-18.5	-18.5	0.44 (1)		10.00
O-N	0 / 1725	-18.5	-18.5	0.32 (1)		10.00
N-M	0 / 1958	-18.5	-18.5	0.32 (1)		10.00
M-L	0 / 2039	-18.5	-18.5	0.45 (1)		10.00
L-K	0 / 0	-18.5	-18.5	0.24 (4)		10.00
K-J	0 / 0	-96.5	-96.5	0.16 (1)		10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R-S	-1871 / 0	0.27 (1)
B-Q	0 / 2062	0.46 (1)
Q-C	0 / 279	0.06 (1)
C-P	-789 / 0	0.39 (1)
P-D	0 / 282	0.07 (4)
D-O	-790 / 0	0.66 (1)
O-E	0 / 525	0.12 (1)
E-N	0 / 66	0.01 (1)
N-F	-113 / 0	0.09 (1)
N-G	0 / 1461	0.33 (1)
M-G	-957 / 0	0.37 (1)
M-H	-623 / 0	0.84 (1)
L-H	-70 / 90	0.03 (4)
A-R	0 / 1602	0.35 (1)
L-I	0 / 2050	0.46 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	OL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.28")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 611 (0.03")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 304 (0.06")

CSI: TC=0.61/1.00 (H-I:1) , BC=0.57/1.00 (P-Q:1) , WB=0.84/1.00 (H-M:1) , SSI=0.23/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (R) (INPUT = 0.90)
JSI METAL= 0.69 (I) (INPUT = 1.00)

A-18023190

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292577	J1T	4	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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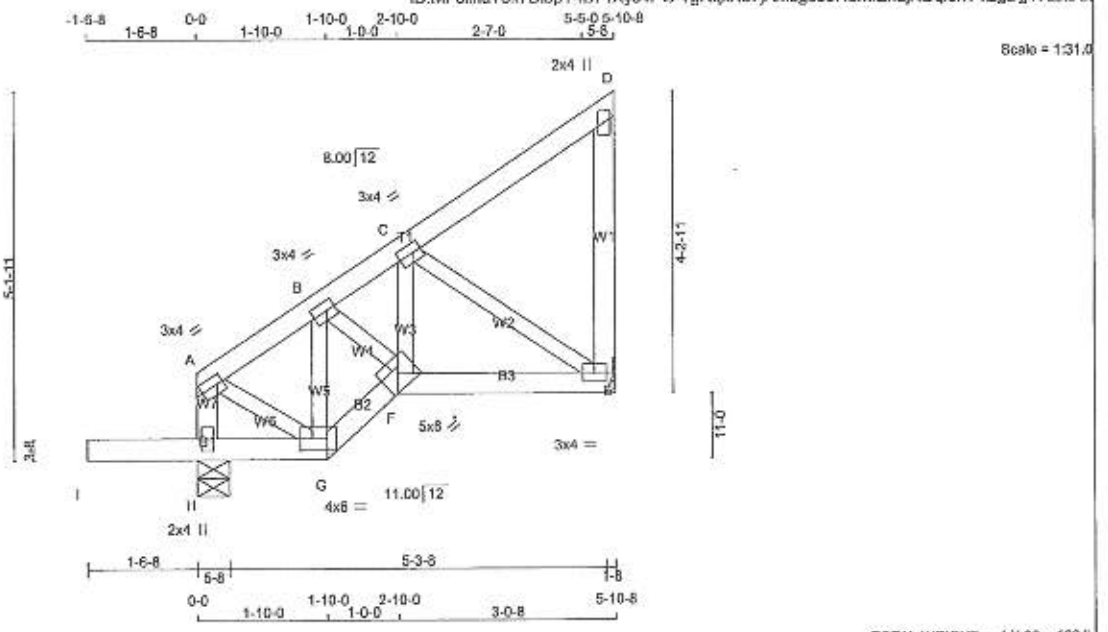

TOTAL WEIGHT = 4 X 30 = 120 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
H - A	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-I	MT20	3.0	4.0	1.50	1.00
B	TMWW-I	MT20	3.0	4.0	1.50	1.50
C	TMWW-I	MT20	3.0	4.0	1.50	1.50
D	TMV+p	MT20	2.0	4.0		
E	BMWW1-I	MT20	3.0	4.0		
F	BBWW-b	MT20	5.0	6.0		
G	BBWW-I	MT20	4.0	6.0	2.00	4.50
H	BMV1+p	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	452	0	452	0	0	5-8	1-8
E	264	0	264	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	322	197 / 0	0 / 0	0 / 0	0 / 0	125 / 0	0 / 0
E	188	115 / 0	0 / 0	0 / 0	0 / 0	73 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H-A	-223 / 0	0.0	0.0	0.02 (1)	7.81	A-G	0 / 154	0.03 (1)
A-B	-165 / 0	-78.0	-78.0	0.04 (1)	6.25	G-B	-220 / 0	0.03 (1)
B-C	-239 / 0	-78.0	-78.0	0.08 (1)	6.25	B-F	0 / 130	0.03 (1)
C-D	-11 / 0	-78.0	-78.0	0.05 (1)	6.25	F-C	0 / 83	0.02 (4)
E-D	-95 / 0	0.0	0.0	0.03 (1)	7.81	C-F	-259 / 0	0.06 (1)
I-H	0 / 0	-96.5	-96.5	0.16 (1)	10.00			
H-G	0 / 0	-18.5	-18.5	0.15 (1)	10.00			
G-F	0 / 173	-18.5	-18.5	0.03 (1)	10.00			
F-E	0 / 216	-18.5	-18.5	0.07 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 588 (0.03")

CSI: TC=0.08/1.00 (B-C:1) , BC=0.16/1.00 (G-H:1) , WB=0.06/1.00 (C-E:1) , SSI=0.12/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.50 (A) (INPUT = 0.90)
JSI METAL= 0.08 (E) (INPUT = 1.00)

A-18023191

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292577	J1TA	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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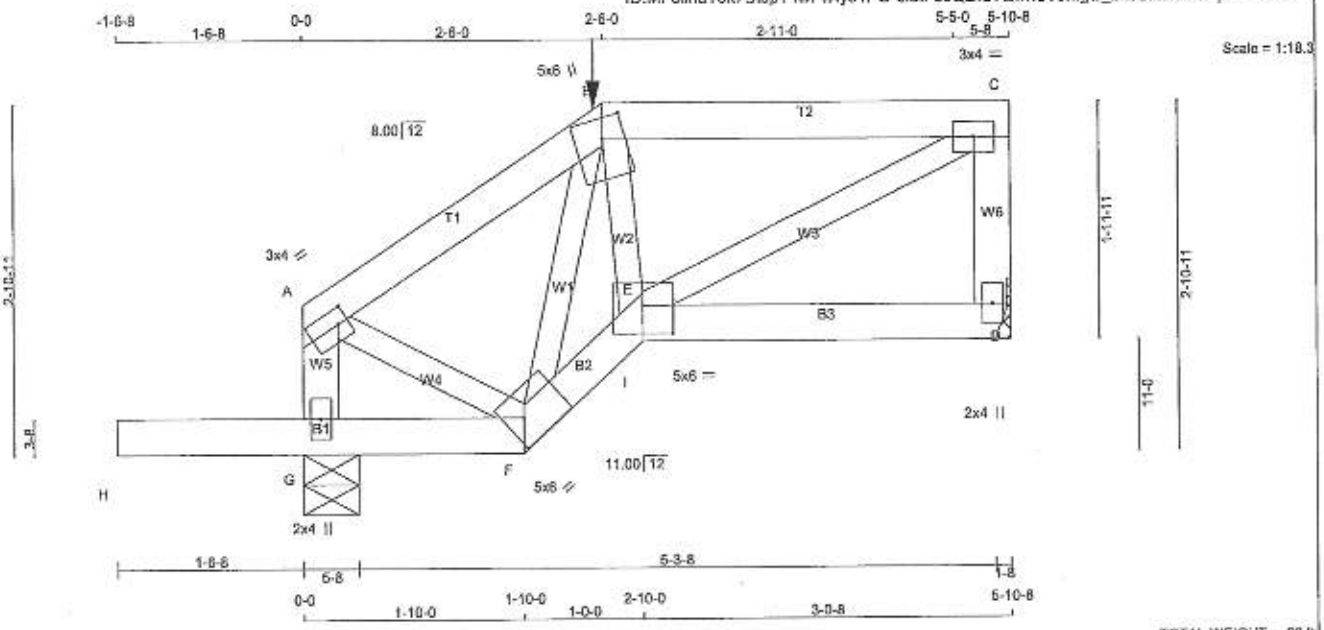

TOTAL WEIGHT = 28 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
G - A	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-t	MT20	3.0	4.0	1.50	1.00
B	TTWW+m	MT20	5.0	6.0	2.00	2.25
C	TMWW-t	MT20	3.0	4.0		
D	BMV1+p	MT20	2.0	4.0		
E	BBWW-t	MT20	5.0	6.0	2.75	3.00
F	BBWW-h	MT20	5.0	6.0	2.50	1.75
G	BMV1+p	MT20	2.0	4.0		

HANGERS NOTES
1] SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 71.7 lbs FACTORED DOWN AT 2-6-0 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	398	0	398	0	0	HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	
G	548	0	548	0	0	5.8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	283	174 / 0	0 / 0	0 / 0	0 / 0	109 / 0	0 / 0
G	391	236 / 0	0 / 0	0 / 0	0 / 0	155 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-226 / 0	-78.0	-78.0	0.09 (1)	6.25	F- B	-389 / 0	0.07 (1)
B- C	-314 / 0	-111.3	-111.3	0.24 (1)	6.25	B- E	0 / 147	0.04 (1)
D- C	-358 / 0	0.0	0.0	0.04 (1)	7.81	E- C	0 / 358	0.09 (1)
G- A	-313 / 0	0.0	0.0	0.03 (1)	7.81	A- F	0 / 207	0.05 (1)
H- G	0 / 0	-96.5	-96.5	0.17 (1)	10.00			
G- F	0 / 0	-26.4	-26.4	0.17 (1)	10.00			
F- I	0 / 387	-26.4	-26.4	0.07 (1)	10.00			
I- E	0 / 387	-26.4	-26.4	0.07 (1)	10.00			
E- D	0 / 0	-26.4	-26.4	0.08 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
B	2-6-0	-72	-72	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: GPrimeHip
LEFT SETBACK = 2-6-0
RIGHT SETBACK = 0-0
END SETBACK = 3-8-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 624 (0.03")

CSI: TC=0.24/1.00 (B-C:1) , BC=0.17/1.00 (F-G:1) , WB=0.09/1.00 (C-E:1) , SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

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CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292577	J1TB	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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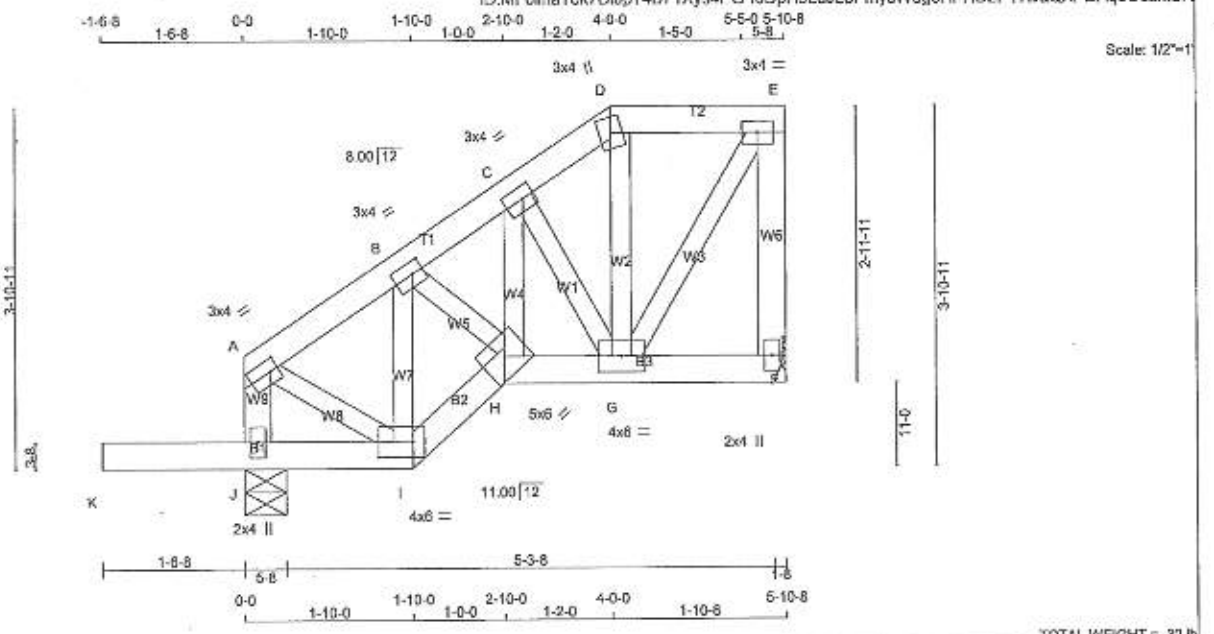

TOTAL WEIGHT = 32 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - A	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	3.0	4.0	1.50	1.00
B	TMWW-t	MT20	3.0	4.0	1.50	1.50
C	TMWW-t	MT20	3.0	4.0	1.50	1.50
D	TTW-m	MT20	3.0	4.0		
E	TMVW-t	MT20	3.0	4.0		
F	BMV1+p	MT20	2.0	4.0		
G	BWWW-t	MT20	4.0	6.0		
H	BBWW-h	MT20	6.0	6.0		
I	BBWW-t	MT20	4.0	6.0	2.00	4.50
J	BMV1+p	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	264	0	264	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
J	452	0	452	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	188	115 / 0	0 / 0	0 / 0	0 / 0	73 / 0	0 / 0
J	322	197 / 0	0 / 0	0 / 0	0 / 0	125 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	-169 / 0	-78.0	-78.0	0.03 (1)	6.25
B-C	-237 / 0	-78.0	-78.0	0.02 (1)	6.25
C-D	-144 / 0	-78.0	-78.0	0.01 (1)	6.25
D-E	-116 / 0	-78.0	-78.0	0.05 (1)	6.25
F-E	-248 / 0	0.0	0.0	0.04 (1)	7.81
J-A	-223 / 0	0.0	0.0	0.02 (1)	7.81
K-J	0 / 0	-96.5	-96.5	0.16 (1)	10.00
J-I	0 / 0	-18.5	-18.5	0.16 (1)	10.00
I-H	0 / 179	-18.5	-18.5	0.03 (1)	10.00
H-G	0 / 188	-18.5	-18.5	0.03 (1)	10.00
G-F	0 / 0	-18.5	-18.5	0.01 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FORCE (LBS)	MAX CSI (LC)
I-B	-227 / 0	0.04 (1)
B-H	0 / 86	0.02 (1)
H-C	0 / 84	0.02 (1)
C-G	-138 / 0	0.02 (1)
G-D	-32 / 7	0.01 (1)
G-E	0 / 211	0.05 (1)
A-I	0 / 159	0.04 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 588 (0.03")

CSI: TC=0.05/1.00 (D-E:1) , BC=0.16/1.00 (I-J:1) , WB=0.05/1.00 (E-G:1) , SSI=0.12/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.52 (A) (INPUT = 0.90)
JSI METAL= 0.08 (A) (INPUT = 1.00)

A-18023193

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292577	J1TC	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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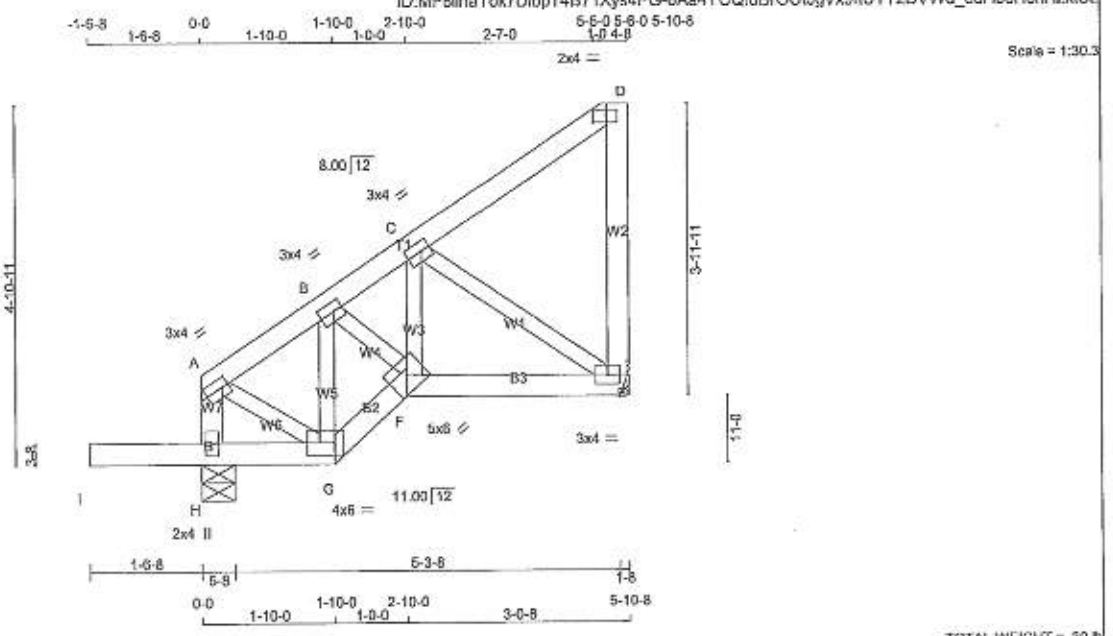


TOTAL WEIGHT = 30 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-t	MT20	3.0	4.0	1.50	1.00
B	TMWW-t	MT20	3.0	4.0	1.50	1.50
C	TMWW-t	MT20	3.0	4.0	1.50	1.50
D	TVM-p	MT20	2.0	4.0	0.50	2.25
E	BMVW1-t	MT20	3.0	4.0		
F	BBWW-h	MT20	5.0	6.0		
G	BBWW-t	MT20	4.0	6.0	2.00	4.50
H	BMV1+p	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN SX	REQRD BRG IN-SX
E	264	0	264	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
H	452	0	452	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	188	115 / 0	0 / 0	0 / 0	0 / 0	73 / 0	0 / 0
H	322	197 / 0	0 / 0	0 / 0	0 / 0	125 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (1)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX. UNBRAC LENGTH
A- B	-165 / 0	-78.0	-78.0	0.04 (1)	6.25
B- C	-239 / 0	-78.0	-78.0	0.06 (1)	6.25
C- D	-11 / 0	-78.0	-78.0	0.06 (1)	6.25
E- D	-85 / 0	0.0	0.0	0.03 (1)	7.81
H- A	-223 / 0	0.0	0.0	0.02 (1)	7.81
I- H	0 / 0	-96.5	-96.5	0.16 (1)	10.00
H- G	0 / 0	-18.5	-18.5	0.16 (1)	10.00
G- F	0 / 173	-18.5	-18.5	0.03 (1)	10.00
F- E	0 / 216	-18.5	-18.5	0.07 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- B	-220 / 0	0.03 (1)
B- F	0 / 130	0.03 (1)
F- C	0 / 63	0.02 (4)
C- E	-259 / 0	0.06 (1)
A- G	0 / 154	0.03 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 588 (0.03")

CSI: TC=0.06/1.00 (B-C:1) , BC=0.16/1.00 (G-H:1) , WB=0.06/1.00 (C-E:1) , SSI=0.12/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	364	1687	788	1987	1656

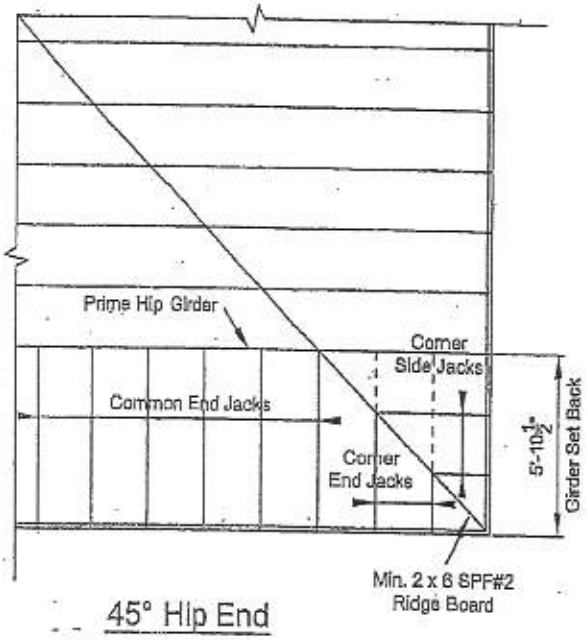
PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.50 (A) (INPUT = 0.90)
JSI METAL= 0.06 (D) (INPUT = 1.00)

A-18023194

STRACON ENGINEERING INC.



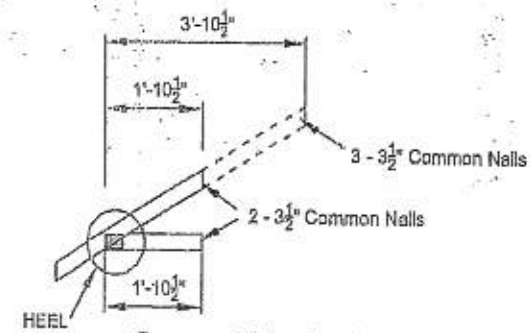
45° Hip End

LUMBER SPECIFICATION

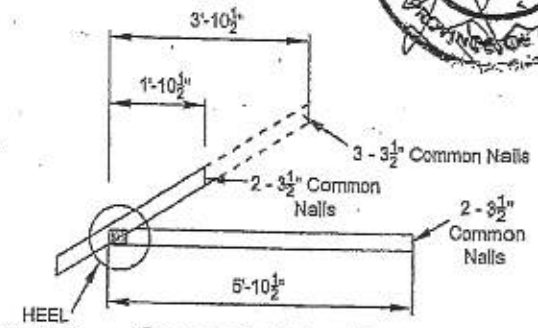
TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

DESIGN LOAD

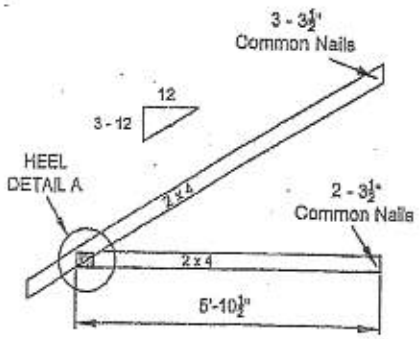
TOP CHORD SNOW LOAD	:	40.5 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD	:	50.5 P.S.F



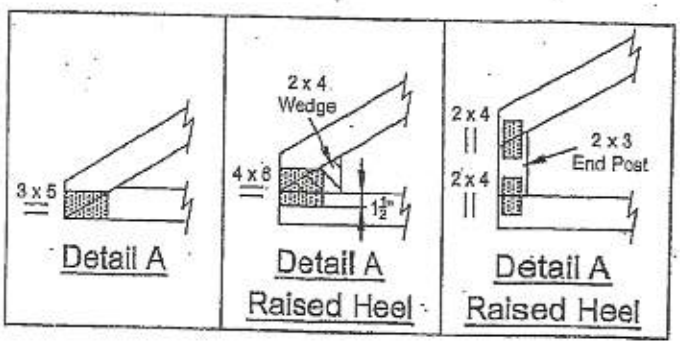
Corner Side Jacks



Corner End Jacks



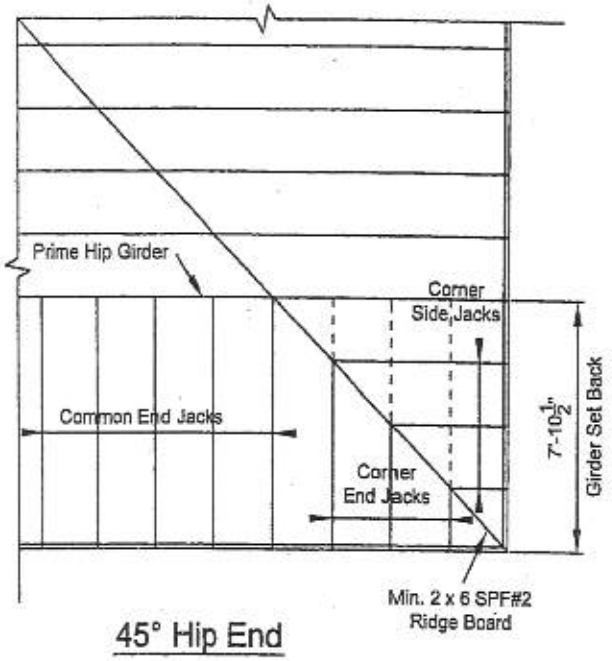
Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

CS-51008

STRACON ENGINEERING INC.



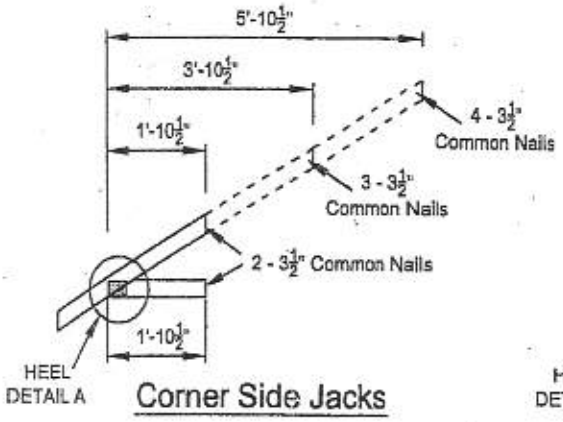
45° Hip End

LUMBER SPECIFICATION

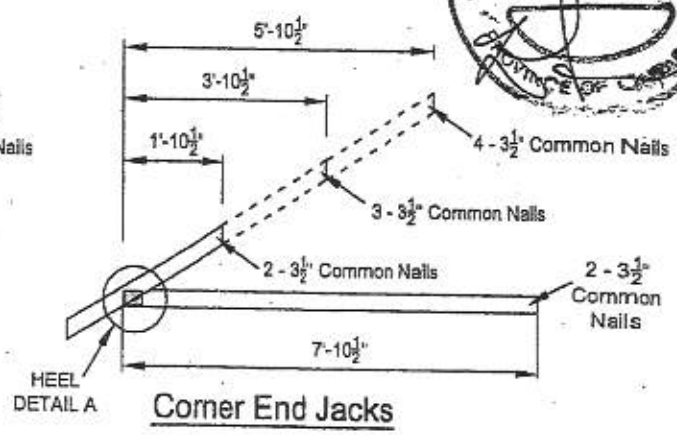
TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

DESIGN LOAD

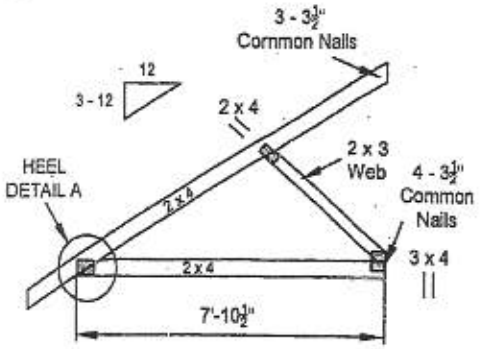
TOP CHORD SNOW LOAD	:	34.8 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD	:	44.8 P.S.F



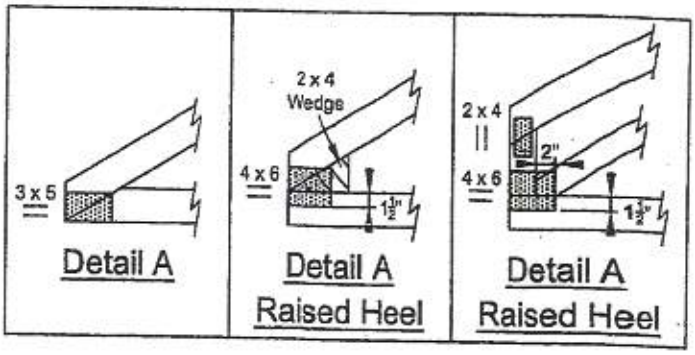
Corner Side Jacks



Corner End Jacks



Common End Jacks



Detail A | Detail A Raised Heel | Detail A Raised Heel

NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

CS-71008N

TECHNICAL BULLETIN

LUS – Double Shear Joist Hangers

SIMPSON Strong-Tie

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

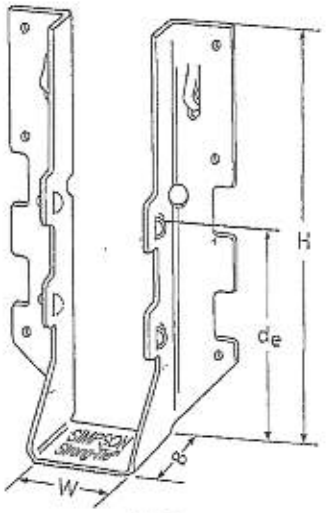
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

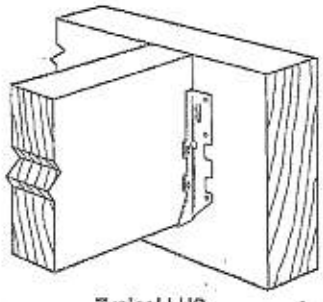
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified

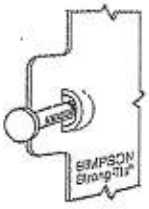


LUS28

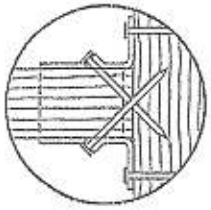

Typical LUS Installation

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LUS24	18	1 9/16	3 1/8	1 3/4	1 15/16	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 15/16	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1 9/16	4 3/4	1 3/4	3 5/8	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 7/8	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4 5/8	4 5/8	2	3 1/4	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1 9/16	6 5/8	1 3/4	3 3/4	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4 5/8	6 1/4	2	3 1/4	(6) 16d	(4) 16d	1720	3326	1545	2375
LUS210	18	1 9/16	7 13/16	1 3/4	3 7/8	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4 5/8	8 9/16	2	5 1/4	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 6,603,680


Double Shear Nailing Top View.

LICENSED PROFESSIONAL ENGINEER
4/5/2017
B. D. BUNTING
PROVINCE OF ONTARIO

UNITED STATES DESIGN

T-SPECLUS17 6/17 Exp. 8/19

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

TC – Truss Connectors

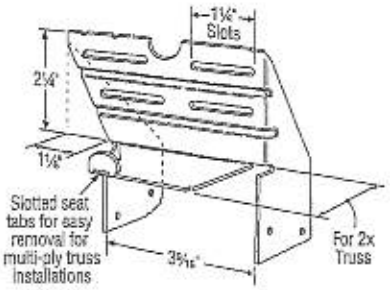
SIMPSON Strong-Tie

The TC truss connector is an ideal connector for scissor trusses and can allow horizontal movement up to 1¼". The TC also attaches plated trusses to top plates or sill plates to resist uplift forces. Typically used on one or both ends of truss as determined by the building designer.

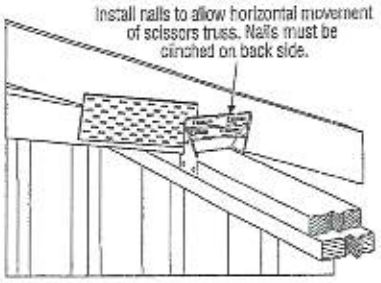
Material: 16 gauge

Finish: G90 galvanized

Design: Factored resistances are in accordance with CSA 086-14



TC24
U.S. Patent 4,932,173



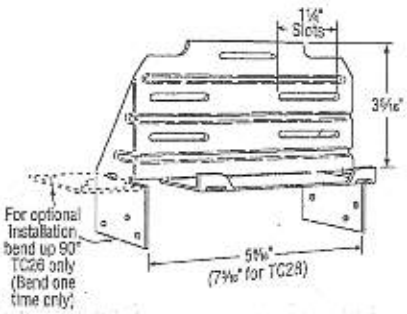
Typical TC24 Installation

Installation:

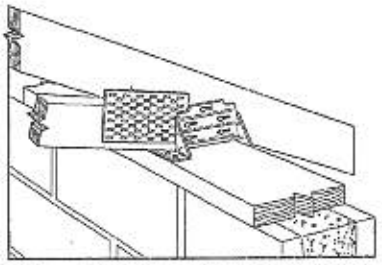
- Use all specified fasteners.
- Nails: 10d = 0.148" dia. x 3" long common wire, 10d x 1½ = 0.148" dia. x 1½" long.
- Drive 10d nails into the truss at the inside end of the slotted holes (inside end is towards the centre of the truss) and clinch on the back side. Do not seat these nails into the truss–allow room under the nail head for movement of the truss with respect to the wall.

Optional TC Installation:

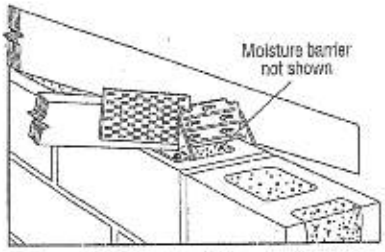
- Bend one flange up 90°. Drive specified nails into the top and face of the top plates or install Titen® screws into the top and face of masonry wall. See optional load tables and installation details.



TC26
(TC28 Similar)



Optional TC26 Installation for Grouted Concrete Block using a Wood Nailer (8", 10", 12" Wall Installation Similar)



Optional TC26 Installation for Grouted Concrete Block using Titen Screws

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K_D=1.15)	Uplift (K_D=1.15)
			lb.	lb.
TC24	(4) 10d	(4) 10d	605	430
TC26	(5) 10d	(6) 10d	1015	720
TC28	(5) 10d	(6) 10d	1015	720

Optional TC Installation Table

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K_D=1.15)	Uplift (K_D=1.15)
			lb.	lb.
TC26	(5) 10d	(6) 10d x 1½"	810	660
	(5) 10d	(6) 10d	930	660

1. Factored resistances have been increased 15% for earthquake or wind loading; no further increase allowed; reduce where other loads govern.
2. Grout strength is 15 MPa minimum.
3. Optional TC26 installation with 10d nails requires minimum 3" top plate thickness.
4. TC26 fastened to grouted concrete block with (6) – ⅜" x 2¼" Titen screws has a factored uplift resistance of 275 lb.

LICENSED PROFESSIONAL ENGINEER
4/3/2017
B. D. BUNTING
PROVINCE OF ONTARIO

LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2019 and reflects information available as of March 1, 2017. This information is updated periodically and should not be relied upon after June 30, 2019. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

T-SPECTC17 3/17 exp. 6/19

(800) 999-5099
strongtie.com