



Job Track: 45147
Layout ID: 292596
Plan Log: 95914

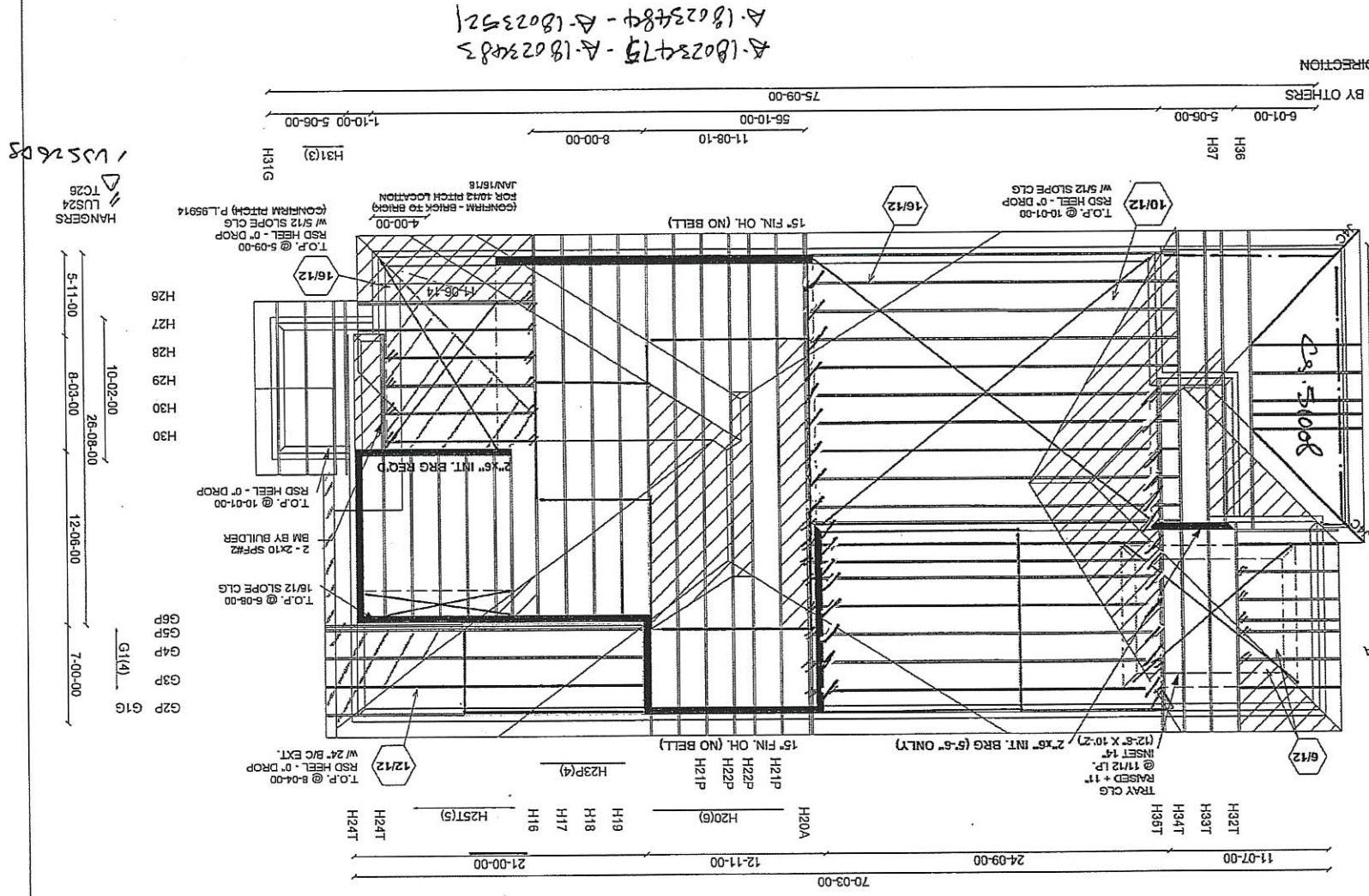
Builder / Location: GOLD PARK HOMES / VAUGHAN
Project: PINE VALLEY
Date: 1/17/2018
Designer: AMANDA

Model / Elevation: 4201 REV1 / B LOGGIA W/TRAY
THESE DRAWINGS CONSTITUTE THE PROPERTY OF ALPA ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY ALPA ROOF TRUSSES INC AND WILL BE RETRACTED BY ALPA ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. (Latest Edition)
ROOF RAFTERS THAT CROSS MEET WITH A VERT. POST TO THE TRUSS UNDEREATH AT EACH CROSS PT. TO HAVE LATERAL BRACING SO THAT THE DIST. BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

H10G H11P H12P H13P H14P H16P(4) H10T(5) H9T(7) H9T H8T H7T H6T H5T
15' B/C EXT.
15' RSD, HEEL - 0" DROP (2" x 6" FASCIA) - FM ASPHALT SHINGLES 2" x 6" BRG/BRICK PIGGYBACK TRS PURLINS BY OTHERS LOFTS WALLS SUPPORTED BY OTHERS

CONFIRM TRUSS LAYOUT/DIRECTION
P.L. 95914



CONVENTIONAL FRAMING BY OTHERS

ALPA-BARRIE

Stracon Engineering Inc.

69 Graydon Crescent
Richmond Hill, Ontario
L4B 3W7
(905) 832-2250 Fax (905) 832-0286



RESPONSIBILITIES

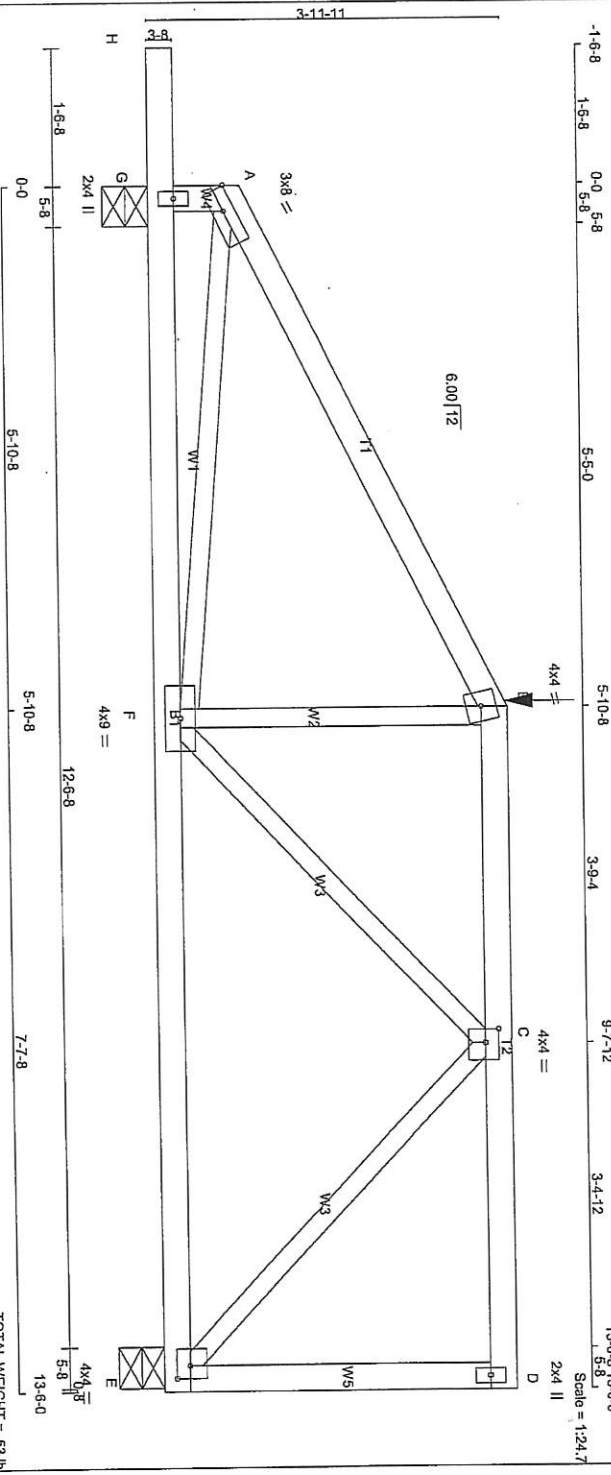
1. Stracon Engineering Inc. is responsible for the design of trusses as individual components.
2. It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities with jurisdictions.
3. All dimensions are to be verified by owner contractor, architect or other authority before manufacture.
4. Stracon Engineering Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Stracon Engineering Inc. drawings is specified for the truss as a component only and forms an integral part of the truss design.
5. It is the truss manufacturer's responsibility to ensure that trusses are manufactured in conformance with Stracon Engineering Inc. specifications outlined below.

SPECIFICATIONS

1. Trusses designed by Stracon Engineering Inc. conforms to the relevant section of the Ontario Building Code of Canada (Part 9 or Part 4) or to the Canadian code for farm buildings, whichever applies to the building type indicated on the drawings as well as to the procedures established by the truss plate institute of Canada. Unit stresses used are as per CSA-086-09.
2. Lumber is to be the sizes and grade specified.
3. Moisture content of lumber is not to exceed 19% in service unless otherwise specified.
4. Lumber not to be treated with chemicals unless otherwise specified.
5. Plates shall be applied to both faces of the truss at each joint and shall be positioned as specified.
6. The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals not exceeding 12.5 times in thickness.
7. Where not rigid ceiling is attached directly to the bottom chord, laterally brace the chords at intervals not exceeding 3M (10') o.c.

January 15, 2014

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292591	H32	1	1		
Alpha Roof Truss, Maple					
Version 8.200 S Jan 6 2018 MITek Industries, Inc. Sat Feb 17 12:49:11 2018 Page 1					ID:MF8l1iaToK7DlbpT4871Xys4FG-LZFHXv69qZ1_KCZe6K4P10v17lBkKqjKXyXZkENC
13-0-8 13-6-0					13-0-8 13-6-0
Scale = 1:24.7					3-4-12



LUMBER	N.L.G.A. RULES	LUMBER	DESCR.
CHORDS	SIZE		
A - B	2x4	No.2	SPF
B - C	2x4	No.2	SPF
C - D	2x4	No.2	SPF
D - E	2x4	No.2	SPF
E - F	2x4	No.2	SPF
F - G	2x4	No.2	SPF
G - H	2x4	No.2	SPF
H - I	2x4	No.2	SPF
ALL WEBS	2x3	DRY	SPF
		No.2	SPF

[illegible][illegible][illegible]

BEARING SIZE FACTOR (R = 1.0741 UNIT) (S) (TABLED ON CD) (TABLED ON CD)	DESIGN CONSISTS OF <u>2</u> TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:
BRACING	
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.70 FT.	
CEILING TO BE SHEATHED OR MAX. PURLIN SPACING = 2.70 FT.	
CEILING CHORD LENGTHS = 11.00 FT. OR RIGID CEILING DIRECTLY TO CEILING CHORDS	
END WALL WIDTH = 0-0	
END DISTANCE = 23-10-0	
END SPAN CARRIED = 25-5-1	
START SPAN CARRIED = 25-6-1	

CHORDS #ROWS	SURFACE	LOAD(PLF)	APPLIED.
MAX. UNBRAIDED BOTTOM CHORD ELEMENT = 1088' 3" - 2' 0" = 1086' 3"			
APPLIED TO FRONT SIDE OF BOTTOM CHORD. -ADDT'L LOADS BASED ON 55 % OF GSL.			

SPACING (IN)		ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.		THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC2010, NBC2015	
TOP CHORDS : (0.122'x3') SPIRAL NAILS					
A-C	1	12	TOP		
C-E	1	12	TOP		
E-F	1	12	TOP		
F-G	2	12	TOP		
BOTTOM CHORDS : (0.122'x3') SPIRAL NAILS					
M-J	2	9	SIDE(2x2.4)		
J-K	2	9	SIDE(2x2.4)		
WEBS : (0.122'x3') SPIRAL NAILS					
K-L	1	6	END VERTICALS) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW		
2x8	2	6	END VERTICALS) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW		
			LOADING		

LOADING TOTAL LOAD CASES: (4)

ROOF LIVE LOAD

[illegible][illegible]

	RED PROFESSIONAL ENGINEER	02/17/2014
N-E	0 / 10163	-583.4 -583.4 0.78 (1)
L-K	0 / 10160	-583.4 -583.4 0.61 (1)
K-J	0 / 6580	-583.4 -583.4 0.81 (1)
J-I	0 / 6580	-583.4 -583.4 0.57 (1)
I-H	0 / 2790	-583.4 -583.4 0.24 (1)
H-G	0 / 0	-583.4 -583.4 0.24 (1)

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVEHEELS OFF

TRUSS PLATE MANUFACTURER IS NOT

RESPONSIBLE FOR QUALITY CONTROL IN
THE TRUSS MANUFACTURING PLANT.

MAIL VALUES

PLATE GRIP(DRY) (PSI)	SHEAR (PL)	SECTION (PL)

	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656



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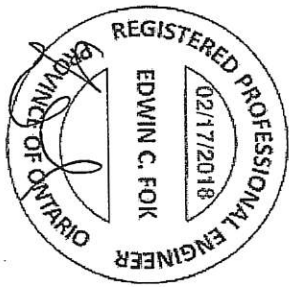
PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TM6MW+1 MT20	6.0	12.0	3.25	3.25
B	TM6MW+1 MT20	6.0	9.0	2.25	3.25
C	TTW-m MT20	5.0	8.0	1.75	4.00
D	TM6W+1 MT20	4.0	4.0	2.00	1.75
E	TTWV+m MT20	6.0	9.0	Edge	4.50
F	TM6W+P MT20	5.0	8.0	Edge	2.75
G	BMV+H MT20	6.0	9.0	Edge	0.50
H	BM6W+1 MT20	4.0	9.0	3.75	1.50
I	BM6W+1 MT20	5.0	8.0	4.00	1.50
J	BS-1 MT20	5.0	6.0		
K	BM6WV+1 MT20	7.0	8.0	4.25	2.75
L	BM6W+W MT20	3.0	8.0	4.25	1.50

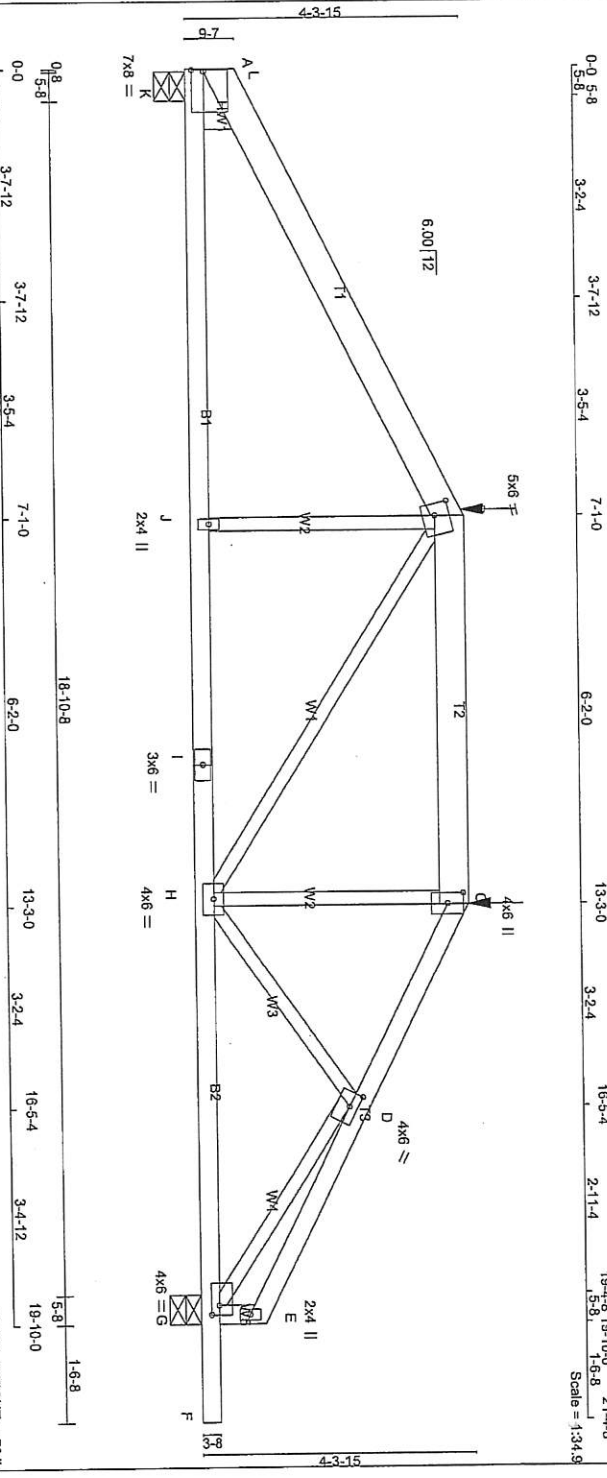
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.
JSI GRP = 0.90 (E) (IN PUT = 0.90)
JSI METAL = 0.83 (U) (IN PUT = 1.00)

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



D.18023474(2)



LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER			
N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.	BEARINGS	FACTORED	MAXIMUM FACTORED
A - B	2x6	DRY	No.2	SPF	VERT	GROSS REACTION	MAXIMUM FACTORED
B - C	2x6	DRY	No.2	SPF	HORIZ	DOWN	BRG
C - E	2x4	DRY	No.2	SPF	HORIZ	UP/LIFT	IN-SX
G - I	2x4	DRY	No.2	SPF	HORIZ	UP/LIFT	IN-SX
I - F	2x4	DRY	No.2	SPF	HORIZ	UP/LIFT	IN-SX
ALL WEBS	2x3	DRY	No.2	SPF	HORIZ	UP/LIFT	IN-SX
EXCEPT							
DRY: SEASONED LUMBER.							

PLATES (table is in inches)				UNFACTORED REACTIONS			
JT TYPE	PLATES	W	LEN	Y	X	IST CASE	SNOW
A - TBH1-1	MT20	7.0	8.0	2.25	0.26	COMBINED	801/0
B - TTW1-m	MT20	5.0	6.0	2.75	2.26	LIVE	0/0
C - TTW1-p	MT20	4.0	6.0	3.00	2.00	PERMALIVE	0/0
D - TWW1-1	MT20	4.0	6.0	1.50	2.75	WIND	0/0
E - TWW1-p	MT20	2.0	4.0	1.50	2.75	DEAD	544/0
G - BWV1-1	MT20	4.0	6.0	1.50	1.75	SOIL	0/0
H - BWV1-1	MT20	3.0	6.0				589/0
I - BS-1	MT20	2.0	4.0				0/0
J - BWV1-m	MT20	2.0	4.0				0/0

HANGERS NOTES				LOADING			
1) SPECIAL HANGER(S) OR CONNECTION(S)				MEMB.	MAX. FACTORED	FACTORED	W EBS
REQUIRED TO SUPPORT CONCENTRATED				FORCE	VERT. LOAD/LC1	MAX. UNBRACED	MEMB.
LOAD(S) 434.8 lbs FACTORED DOWN AT 13-3-0,				(lbs)	(PLF)	CS1 (C)	FORCE
AND 467.9 lbs FACTORED DOWN AT 7-1-0 ON							(lbs)
TOP CHORD. DESIGN FOR UNSPECIFIED							CS1 (C)
CONNECTION(S) IS DELEGATED TO THE							MAX
BUILDING DESIGNER.							CS1 (C)

FACTORED CONCENTRATED LOADS (lbs)				W EBS			
JT	LOC.	LC1	MAX	MAX	MAX	MAX	MAX
A	7-1-0	468	468	468	468	468	468
B	13-3-0	435	435	435	435	435	435
C							

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.2.3.13.11, and no less than 2x4 for Part 4 design



Item	Mean Similarity Score
0.0 5.8	0.0
4.2 4	4.2
4.7 12	4.7
4.5 4	4.5
9.1 0	9.1
2.2 0	2.2
11.3 0	11.3
4.2 4	4.2
15.5 4	15.5
3.1 4	3.1
19.4 8	19.4
19.10 0	19.1
21.4 8	21.4

Scale = 0.0 to 21.48



BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.45 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.	PART 9, NBCC 2010, NBCC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF CBC 2012, CBC 2018 - CSA 086-09, CSA 086-14
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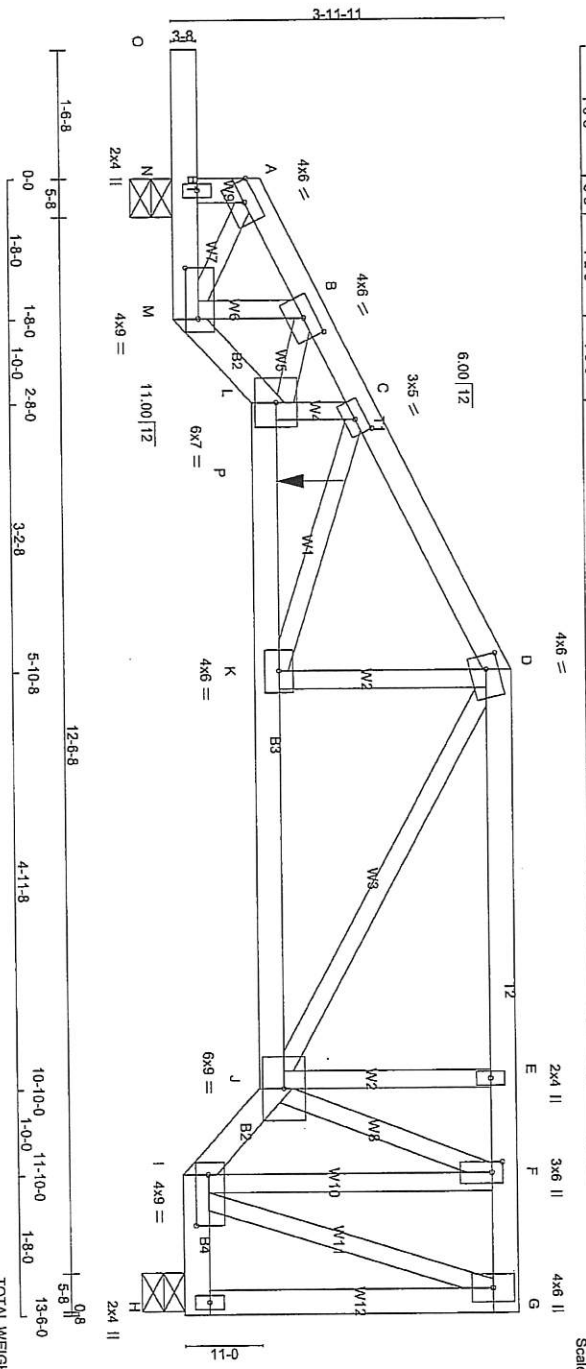
NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRAWG NO.
292592	H32T	1	1	TRUSS DESC.	
Alpa Roof Truss, Maple					

Version 8.200 S Jan 6 2018 MITek Industries, Inc. Sat Feb 17 13:05:17 2018 Page 1
ID:MF-BilinaToK7Dl0pT4B7IXs4PG-3pdxGd0SkriwfmWnc6z0A4y4p31U50X3cxzEgW

Scale = 1:20.2



TOTAL WEIGHT = 62 lb

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
N - A	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
DRY, SEASONED LUMBER.				

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
GROSS REACTION	DOWN	GROSS REACTION	BRG	BRG
VERT	1341	DOWN	UP/LIFT	IN-SX
H	1448	0	0	5-8
N	1027	657/0	0/0	0/0
H	956	595/0	0/0	361/0
N	1027	657/0	0/0	369/0
H	956	595/0	0/0	361/0
N	1027	657/0	0/0	369/0

DESIGN CRITERIA
*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE
SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 34.4 PSF
SPACING = 24.0 IN. C/C
LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM
GIRDER TYPE: C-Profile
LEFT SETBACK = 5-10-8
RIGHT SETBACK = 0-0
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNERS FRAMING TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
-ADDTL. LOADS BASED ON 55% OF G.S.L. LOADS APPLIED TO FIRST 2-8-0 OF SPAN MEASURED FROM THE RIGHT.
GIRDER TYPE: C-Profile
START DISTANCE = 3-7-4
RIGHT SPAN CARRIED = 5-10-8
END DISTANCE = 10-10-0
END SPAN CARRIED = 5-10-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
-ADDTL. LOADS BASED ON 55% OF G.S.L.
*** NON STANDARD GIRDER ***
ADDTL. USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015
THIS DESIGN COMPLIES WITH:
-PART 9 OF OBC 2012, OBC 2018
-CSA 086-09, CSA 086-14
-TIBC 2011, TIBC 2014
DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.
(55% OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD
ALLOWABLE DEF. (LL) = L/360 (0.45")
CALCULATED VERT. DEF. (LL) = L/999 (0.08")
ALLOWABLE DEF. (TL) = L/360 (0.45")
CALCULATED VERT. DEF. (TL) = L/997 (0.16")
CANTILEVER DEFLECTION:
ALLOWABLE DEF. (LL) = L/120 (0.19")
A-18023477
CONTINUED ON PAGE 2

UNFACTORED REACTIONS	1ST LOOSE	MAX. MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
COMBINED	595/0	LIVE	PERM. LIVE	0/0	0/0
N	1027	657/0	0/0	0/0	0/0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.87 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.
LOADING
TOTAL LOAD CASES: (4)
CHORDS
MAX. FACTORED
FORCE
VERT. LOAD
LC1
MAX.
CSF (LC)
MEMB.
FR-TO
FROM
TO
LENGTH
FR-TO
MEMB.
MAX.
FORCE
MAX.
CSF (LC)
WEBS
MAX. FACTORED
FORCE
MAX.
CSF (LC)
MEMB.
FR-TO
FROM
TO
LENGTH
FR-TO
MEMB.
MAX.
FORCE
MAX.
CSF (LC)

CHORDS	MAX. FACTORED	FORCE	VERT. LOAD	LC1	MAX.	CSF (LC)	MEMB.	FR-TO	FROM	TO	LENGTH	FR-TO	MEMB.	MAX.	FORCE	MAX.	CSF (LC)
A-B	-1380/0	-78.0	-78.0	0.10 (1)	5.41	M-B	-1583/0	0.25 (1)	0.44 (1)	0.14 (1)	0.14 (1)	0.14 (1)	0.14 (1)	0.14 (1)	0.14 (1)	0.14 (1)	0.14 (1)
B-C	-2980/0	-78.0	-78.0	0.20 (1)	4.56	L-C	-903/0	0.21 (1)	0.21 (1)	0.21 (1)	0.21 (1)	0.21 (1)	0.21 (1)	0.21 (1)	0.21 (1)	0.21 (1)	0.21 (1)
C-D	-1995/0	-78.0	-78.0	0.26 (1)	5.59	K-D	-787/0	0.41 (1)	0.41 (1)	0.41 (1)	0.41 (1)	0.41 (1)	0.41 (1)	0.41 (1)	0.41 (1)	0.41 (1)	0.41 (1)
D-E	-1131/0	-78.0	-78.0	0.28 (1)	6.25	D-J	-493/0	0.10 (1)	0.10 (1)	0.10 (1)	0.10 (1)	0.10 (1)	0.10 (1)	0.10 (1)	0.10 (1)	0.10 (1)	0.10 (1)
E-F	-1131/0	-78.0	-78.0	0.28 (1)	6.25	D-J	-493/0	0.10 (1)	0.10 (1)	0.10 (1)	0.10 (1)	0.10 (1)	0.10 (1)	0.10 (1)	0.10 (1)	0.10 (1)	0.10 (1)
F-G	-529/0	-153.5	-153.5	0.12 (1)	7.02	J-E	-1558/0	0.42 (1)	0.42 (1)	0.42 (1)	0.42 (1)	0.42 (1)	0.42 (1)	0.42 (1)	0.42 (1)	0.42 (1)	0.42 (1)
G-H	-1313/0	0.0	0.0	0.33 (1)	7.23	J-F	-1558/0	0.40 (1)	0.40 (1)	0.40 (1)	0.40 (1)	0.40 (1)	0.40 (1)	0.40 (1)	0.40 (1)	0.40 (1)	0.40 (1)
H-A	-1215/0	0.0	0.0	0.13 (1)	7.23	I-G	-1558/0	0.12/6	0.32 (1)	0.32 (1)	0.32 (1)	0.32 (1)	0.32 (1)	0.32 (1)	0.32 (1)	0.32 (1)	0.32 (1)
O-N	0/0	-96.5	-96.5	0.17 (1)	10.00	A-M	0/1327	0.32 (1)	0.32 (1)	0.32 (1)	0.32 (1)	0.32 (1)	0.32 (1)	0.32 (1)	0.32 (1)	0.32 (1)	0.32 (1)
N-M	-1/0	-18.5	-18.5	0.17 (1)	10.00	M-L	0/1327	0.32 (1)	0.32 (1)	0.32 (1)	0.32 (1)	0.32 (1)	0.32 (1)	0.32 (1)	0.32 (1)	0.32 (1)	0.32 (1)
M-L	0/1509	-18.5	-18.5	0.27 (1)	10.00	L-P	0/2829	0.10,00	0.10,00	0.10,00	0.10,00	0.10,00	0.10,00	0.10,00	0.10,00	0.10,00	0.10,00
L-P	0/2829	-18.5	-18.5	0.91 (1)	10.00	P-K	-1120	-1120	0.77 (1)	10.00	10.00	10.00	10.00	10.00	10.00	10.00	10.00
P-K	0/1820	-1120	-1120	0.77 (1)	10.00	K-J	-36.4	-36.4	0.12 (1)	10.00	10.00	10.00	10.00	10.00	10.00	10.00	10.00
K-J	0/1820	-1120	-1120	0.77 (1)	10.00	J-I	-36.4	-36.4	0.12 (1)	10.00	10.00	10.00	10.00	10.00	10.00	10.00	10.00
J-I	0/658	-36.4	-36.4	0.12 (1)	10.00	I-H	0/0	0/0	0.03 (4)	10.00	10.00	10.00	10.00	10.00	10.00	10.00	10.00
I-H	0/0	-36.4	-36.4	0.12 (1)	10.00												

FACTORED CONCENTRATED LOADS (LBS)
JT LOC. LC1 MAX- MAX+ FACE DIR. TYPE
3-7-4 -407 -407 FRONT VERT TOTAL

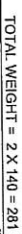
FACTORED CONCENTRATED LOADS (LBS)	JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
	3-7-4	-407	-407	---	---	FRONT	VERT	TOTAL

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



REGISTERED PROFESSIONAL ENGINEER
EDWIN C. FOK
02/17/2018
ONTARIO
A-18023477
CONTINUED ON PAGE 2

Figure 1 is a horizontal bar chart showing the distribution of the number of species per genus (S/G) for 10 genera. The x-axis represents the number of species per genus, ranging from 1 to 10. The y-axis lists the genera: 1-6-8, 1-6-8, 0-0-5-8, 1-8-0, 2-8-0, 1-2-8, 1-0-0, 4-3-15, 6-11-15, 4-3-15, 11-3-14, 5-1-2, 16-5-0, 5-1-2, 21-8-2, 23-4-8, 25-10-0, and 1-10-6. The bars are labeled with the number of species per genus. A scale bar at the bottom indicates a length of 100.



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	IN-PUT BRG	RECORD BRG
VERT	DOWN	HORIZ	UP/LFT
A	8035	0	0
I	7876	0	0

ALLOW FOR 0.3" OF HORIZONTAL MOVEMENT DUE TO TOTAL LOAD.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERMANENT	WIND	DEAD	SOIL
A	5128	3468 / 0	0 / 0	0 / 0	0 / 0	2232 / 0	0 / 0
I	8616	3462 / 0	0 / 0	0 / 0	0 / 0	2188 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, I
BEARING SIZE FACTOR = 1.16 AT JOINT(S) A, I (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2-08 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10-00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x6 DRY SPF No.2 T-BRACE AT E, L
2x4 DRY SPF No.2 T-BRACE AT G, J, H, I

FASTEN T AND L BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 8" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

W E B S

MEMB.	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MAX. UNBRACED LENGTH (FT.)	MEMB. (LBS)	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO	FROM	TO				
5653 / 0	780	780	0.41 / 1	4.28	P-R	-8905 / 0
						0.38 / 1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	DL	=	21.0	PSF
LT	LL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
DL	LL	=	0.0	PSF
TT	LL	=	7.4	PSF
TOTAL LOAD	=	34.4	PSF	

SPACING = 24.0 IN. O.C

LOADING IN PLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GROD TYPE: CS&Gilder
START DISTANCE = 0-0
START SPAN CARRIED = 26-5-1
END DISTANCE = 23-10-0
END SPAN CARRIED = 25-5-1
END WALL WIDTH = 0-0

APPLIED TO FRONT SIDE OF BOTTOM CHORD.
-ADDTL LOADS BASED ON 56% OF GSI.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

-PART 9 OF NBCC 2012, ORC 2018
-CSA 008.9-09, CSA 006-14
-TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(65% OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = U360 (0.79")
CALCULATED VERT. DEFL.(LL) = U999 (0.24")
ALLOWABLE DEFL.(LL) = U360 (0.79")
CALCULATED VERT. DEFL.(LL) = U999 (0.48")



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292592	H35T	1	2		
Alpha Roof Truss, Maple					
Version 8.200 S Jan 6 2018 MITek Industries, Inc. Sat Feb 17 13:07:51 2018 Page 2					
ID:MF8llrlat1ok7Dlbp14871Xys4Pg-ZRW9b1gn3A845Jbvr1lp3Bll_eHlZm9G_rD1ozKEe6					

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
A	TMBMW+1	MT20	10.0	12.0	Edge 4.75
B	TMBMW+1	MT20	5.0	14.0	2.25 4.75
C	TMBWV+1	MT20	4.0	9.0	1.75 4.50
D	TMBWV+1	MT20	5.0	9.0	2.25 2.25
E	TTWV+1m	MT20	4.0	9.0	Edge
F	TMBWV+1	MT20	4.0	4.0	
G	TTWV+1m	MT20	7.0	8.0	Edge 4.75
H	TMBWV+1	MT20	5.0	8.0	Edge 2.75
I	BMV+1+1	MT20	4.0	9.0	Edge 0.50
J	BMVW+1	MT20	4.0	9.0	3.75 1.50
K	BMVW+1	MT20	5.0	10.0	5.00 1.75
L	BBWV+1	MT20	8.0	12.0	4.50 6.00
M	BBWV+1	MT20	8.0	12.0	
N	BBWV+1	MT20	6.0	7.0	3.75 2.25
O	BBWV+1	MT20	10.0	12.0	
P	BBWV+1	MT20	7.0	8.0	3.25 4.00

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

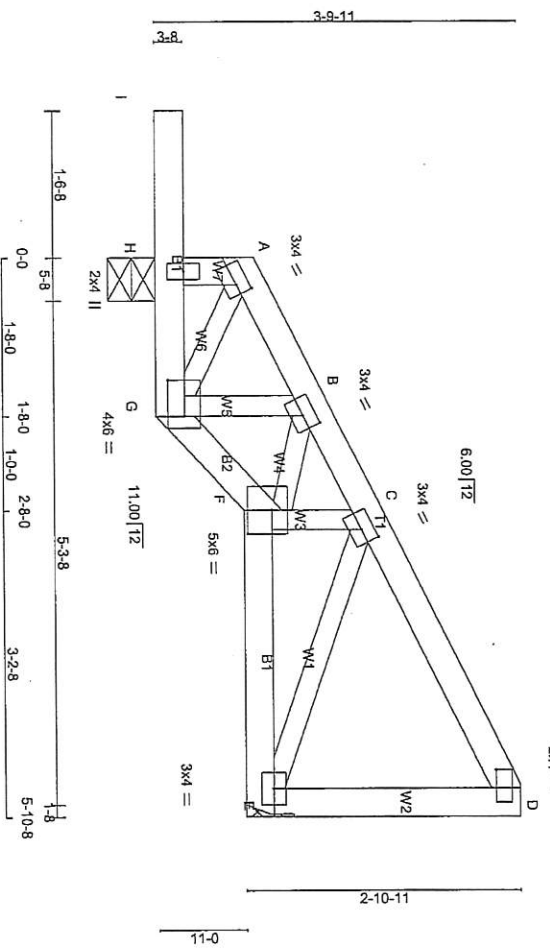
NAIL VALUES
PLATE GRIP(PORV) SHEAR SECTION
(PSI) (PL) (PL)
MAX MIN MAX MIN
MT20 618 354 1667 798 1987 1666
PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.
JSI GRIP = 0.89 (D) (INPUT = 0.80)
JSI METAL = 0.88 (E) (INPUT = 1.00)

NOTE: Lateral brace(s) shown shall be 1x4
for Part 9 design as per OBC 9.23.13.11,
and no less than 2x4 for Part 4 design



A-18023487(C)

Scale = 1/32.2



TOTAL WEIGHT = 28 lb

LUMBER				DESIGN CRITERIA			
N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.	BEARINGS	FACTORED	MAXIMUM FACTORED
A - D	2x4	DRY	No.2	SPF	GROSS REACTION	DOWN	UP/LIFT
E - D	2x4	DRY	No.2	SPF	VERT	0	264
H - A	2x4	DRY	No.2	SPF	HORIZ	0	0
I - G	2x4	DRY	No.2	SPF	HORIZ	0	0
G - F	2x4	DRY	No.2	SPF	HORIZ	0	0
F - E	2x4	DRY	No.2	SPF	HORIZ	0	0
ALL WEBS	2x3	DRY	No.2	SPF	HORIZ	0	0
EXCEPT							
DRY: SEASONED LUMBER.							

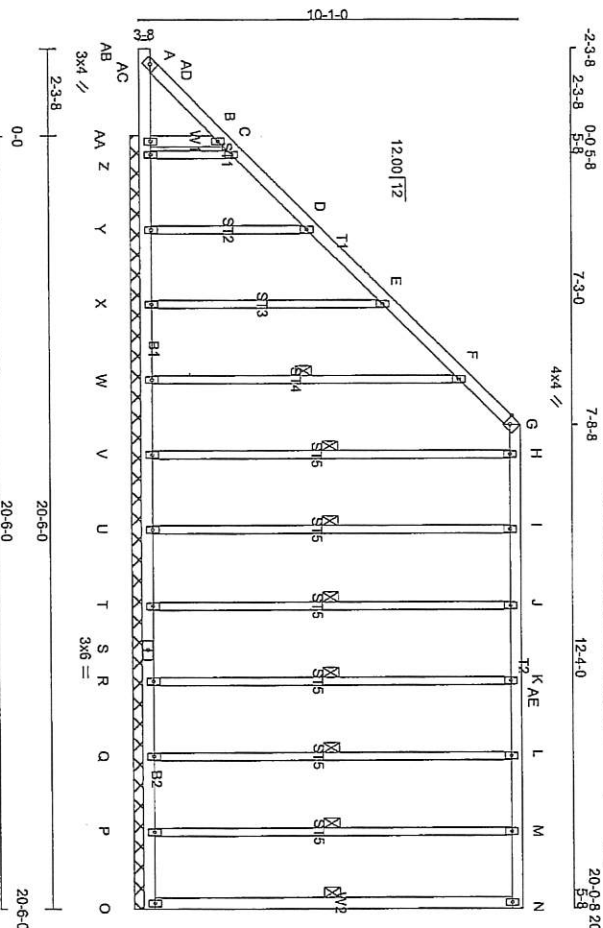
PLATES (table is in inches)				UNFACTORED REACTIONS			
JT TYPE	PLATES	W	LEN Y	X	1ST CASE	MAX MIN COMPONENT REACTIONS	DEAD
A	TMWV-1	M120	3.0	4.0	1.50	1.25	73/0
B	TMWV-1	M120	3.0	4.0	1.50	1.75	73/0
C	TMWV-1	M120	3.0	4.0	1.50	1.75	73/0
D	TMWV-1	M120	3.0	4.0	1.50	1.75	73/0
E	TMWV-1	M120	3.0	4.0	1.50	1.75	73/0
F	TMWV-1	M120	3.0	4.0	1.50	1.75	73/0
G	TMWV-1	M120	3.0	4.0	1.50	1.75	73/0
H	TMWV-1	M120	3.0	4.0	1.50	1.75	73/0

CHORDS				WEBS			
MEMB.	MAX. FACTORED	VERT. LOAD	CS1 (LBS)	MAX. FACTORED	VERT. LOAD	CS1 (LBS)	MAX. FACTORED
FR-TO	FR-TO	FR-TO	FR-TO	FR-TO	FR-TO	FR-TO	FR-TO
A-B	-191/0	-78.0	-78.0	0.03 (1)	8.25	8.25	0.04 (1)
B-C	-376/0	-78.0	-78.0	0.08 (1)	8.25	8.25	0.05 (1)
C-D	-8/0	-78.0	-78.0	0.08 (1)	10.00	10.00	0.02 (4)
E-D	-102/0	0.0	0.0	0.02 (1)	7.81	7.81	0.08 (1)
H-A	-219/0	0.0	0.0	0.02 (1)	7.81	7.81	0.04 (1)
I-H	0/0	-86.5	-86.5	0.16 (1)	10.00	10.00	0.04 (1)
H-G	-1/0	-18.5	-18.5	0.16 (1)	10.00	10.00	0.04 (1)
G-F	0/206	-18.5	-18.5	0.04 (1)	10.00	10.00	0.04 (1)
F-E	0/350	-18.5	-18.5	0.09 (1)	10.00	10.00	0.04 (1)

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.2.3.13.11, and no less than 2x4 for Part 4 design



A-18023483



TOTAL WEIGHT = 133 lb

LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA			
N, L, G, A, RULES	CHORDS	SIZE	LUMBER	DESCR	THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.	THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.	PROVIDE ANCHORAGE AT BEARING JOINT Z FOR 392 LBS FACTORED UPLIFT BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINTS1.	BRACING	FOR SECTION G-N, MAX. PURLIN SPACING = 2.00 FT.	FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.	MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
AA- B	2x4	DRY	No.2	SPF							
A- G	2x4	DRY	No.2	SPF							
G- N	2x4	DRY	No.2	SPF							
O- N	2x4	DRY	No.2	SPF							
AB- S	2x4	DRY	No.2	SPF							
S- O	2x4	DRY	No.2	SPF							
ALL WEBS	2x3	DRY	No.2	SPF							
ALL GABLE WEBS	2x3	DRY	No.2	SPF							
DRY: SEASONED LUMBER.											
GABLE STUDS SPACED AT 2-0-0 OC.											

PLATES (table is in inches)				LOADING				TOTAL LOAD CASES: (4)			
JT TYPE	PLATES	W	LEN Y X	CHORDS	MAX. FACTORED FORCE (LBS)	VERT. LOAD LC1 FROM PLF	MAX. UNBRAC LENGTH FR-TO	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	CS1 (LC)	MAX. CS1 (LC)
A	TM4h	M720	3.0	4.0	7.81	0.0	0.04 (1)	P-M	-187/0	0.13 (1)	0.13 (1)
B	TM4h	M720	2.0	4.0	-78.0	-78.0	0.02 (1)	Q-L	-170/0	0.12 (1)	0.12 (1)
C, D, E, F, H, I, J, K, L, M	TM4h	M720	2.0	4.0	-78.0	-78.0	0.14 (1)	R-K	-173/0	0.12 (1)	0.12 (1)
N	TM4h	M720	2.0	4.0	-78.0	-78.0	0.04 (1)	U-I	-168/0	0.11 (1)	0.11 (1)
O	TM4h	M720	2.0	4.0	-78.0	-78.0	0.04 (1)	V-H	-190/0	0.13 (1)	0.13 (1)
P, Q, R, T, U, V, W, X, Y, Z	TM4h	M720	2.0	4.0	-78.0	-78.0	0.04 (1)	W-F	-168/0	0.12 (1)	0.12 (1)
S	BS-1	M720	2.0	4.0	-85.5	-85.5	0.07 (1)	Z-D	-169/0	0.06 (1)	0.06 (1)
AA	BMV1+P	M720	2.0	4.0	-85.5	-85.5	0.04 (1)	AC-AD	-80/0	0.03 (1)	0.03 (1)

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.2.3.13.1, and no less than 2x4 for Part 4 design



A-18023485

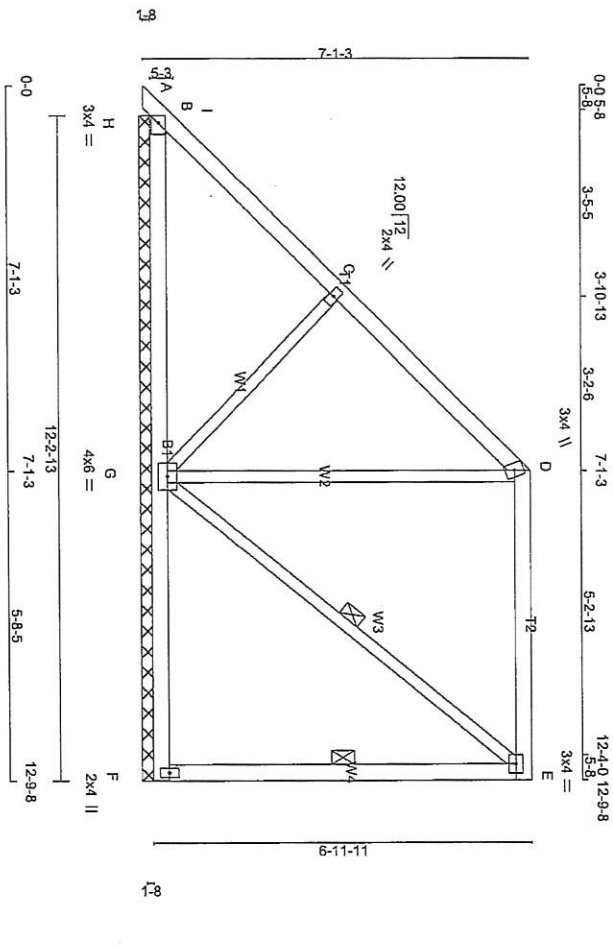
TRUSS DESC

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TOTAL WEIGHT = 49 lb[illegible]

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.1, and no less than 2x4 for Part 4 design





TOTAL WEIGHT = 55 lb

LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER										DESIGN CRITERIA			
N.L.G.A. RULES																	
CHORDS																	
SIZE																	
A - D	2x4	DRY	No.2	DESOR.		BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD			
D - E	2x4	DRY	No.2	SPF	GROSS REACTION	DOWN	HORZ	UP/LIFT	IN-SX	BRG	BRG	IN-SX	DL	6.0	PSF		
F - E	2x4	DRY	No.2	SPF	VERT	0	249	0	12-2-13	1-2-11	1-2-11	1-2-11	DL	0.0	PSF		
B - F	2x4	DRY	No.2	SPF	HORZ	0	338	0	12-2-13	1-2-11	1-2-11	1-2-11	DL	7.4	PSF		
ALL WEBS	2x3	DRY	No.2	SPF	G	0	628	0	0	12-2-13	1-2-11	1-2-11	TOTAL LOAD	= 34.4	PSF		
DRY; SEASONED LUMBER.														SPACING = 24.0 IN. C/C			

UNFACTORED REACTIONS			
1ST LOOSE	MAX MIN.	COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE
F	177	111/0	0/0
B	240	153/0	0/0
G	448	288/0	0/0

FACTORED REACTIONS			
1ST LOOSE	MAX MIN.	COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE
F	177	111/0	0/0
B	240	153/0	0/0
G	448	288/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINTS) F, B, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-F, E-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS			
MEMB.	FACTORED FORCE (LBS)	VERT. LOAD LC1 (PLP)	MAX. UNBRACED LENGTH FR-TO (LBS)
FR-TO	0/11	-78.0	0.01 (1)
A-B	-372/0	-78.0	0.08 (4)
B-I	-196/0	-78.0	0.15 (1)
I-C	-19/0	-78.0	0.14 (1)
C-D	-2/8	-78.0	0.03 (1)
D-E	-212/0	0.0	0.05 (1)

WEBS			
MEMB.	FACTORED FORCE (LBS)	VERT. LOAD LC1 (PLP)	MAX. UNBRACED LENGTH FR-TO (LBS)
FR-TO	0/11	-78.0	0.01 (1)
A-B	-372/0	-78.0	0.08 (4)
B-I	-196/0	-78.0	0.15 (1)
I-C	-19/0	-78.0	0.14 (1)
C-D	-2/8	-78.0	0.03 (1)
D-E	-212/0	0.0	0.05 (1)

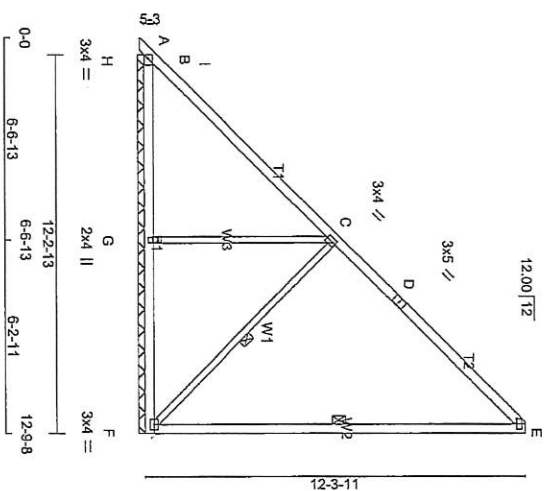
NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
2922593	GEP	1	1	TRUSS DESC.	
Alpha Roof Truss, Maple					

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 ID:MFB8IhaTOK/Dlbp14B71Xys4PG-DFBfZ27NfNAKxrcdUVR62ZUWDL WKThmVmxPEZKEVW
 0.0-5-8 6-1-5 6-6-13 5-9-3 12-4-0 12-5-3 12-9-8
 5-8 12-4-5

Scale = 1/71.2



TOTAL WEIGHT = 59 lb

LUMBER	N. L. G. A. RULES	LUMBER	DESCR.
CHORDS	2x4	No.2	SPF
A - D	2x4	No.2	SPF
D - E	2x4	No.2	SPF
F - E	2x4	No.2	SPF
B - F	2x4	No.2	SPF
ALL WEBS	2x3	No.2	SPF
DRY, SEASONED LUMBER.			

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE				
B TMB1-1	3.0	4.0	1.50	2.25
C TMBW-1	3.0	4.0	1.50	1.00
D TS-1	3.0	5.0		
E TMB-P	2.0	4.0	0.75	2.00
F BMBW1-1	3.0	4.0		
G BMB1+V	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM	FACTORED	INPUT	RECORD
JT	GROSS REACTION	GROSS REACTION	UP/LIFT	IN-SX	IN-SX
F	373	0	0	12-2-13	2-0-7
B	400	0	0	12-2-13	2-0-7
G	442	0	0	12-2-13	2-0-7

UNFACTORED REACTIONS

1ST CASE	MAX/MIN.	COMPONENT REACTIONS	WIND	DEAD	SOIL
JT COMBINED					
F	264	174/0	0/0	0/0	90/0
B	282	189/0	0/0	0/0	93/0
G	319	169/0	0/0	0/0	150/0

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
 ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.
 1 LATERAL BRACES AT 1/2 LENGTH OF E-F, C-F.
 END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

CHORDS					W E B S				
MEMB.	MAX. FACTORED FORCE (lbs)	VERT. LOAD (PLT)	LC1	MAX CS1 (LC)	MEMB.	MAX. FACTORED FORCE (lbs)	MAX CS1 (LC)		
FR-TO	0/11	FROM	0.00	10.00	G-C	-277/0	0.19 (1)		
A-B	-78/15	-78.0	0.09 (1)	6.25	C-F	-207/0	0.10 (1)		
B-I	-178/0	-78.0	0.47 (1)	6.25	H-1	-460/0	0.00 (1)		
I-C	-38/0	-78.0	0.46 (1)	6.25					
C-D	-38/0	-78.0	0.46 (1)	6.25					
D-E	-38/0	-78.0	0.46 (1)	6.25					
F-E	-188/0	0.0	0.17 (1)	6.25					
B-H	0/148	-18.5	0.16 (1)	10.00					
H-G	0/148	-18.5	0.21 (4)	10.00					
G-F	0/148	-18.5	0.21 (4)	10.00					

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

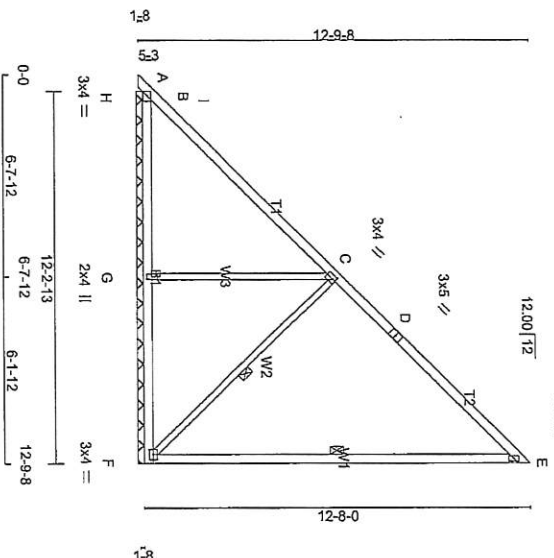


A-18023489

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DWG NO.
292593	G6P	1	1			
Alpa Roof Truss, Maple						

0.0-5.8 5-8
5-8 6-2-4 6-7-12 5-8-4 12-4-0 12-9-8

Scale = 1/12.2



TOTAL WEIGHT = 60 lb

LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N.L.G.A. RULES				
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY, SEASONED LUMBER.				

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED	MAXIMUM FACTORED	INPUT	RECORD
GROSS REACTION	GROSS REACTION	BRG	BRG
VERT	DOWN	IN-SX	IN-SX
JT 373	373	0	12-2-13
F 406	406	0	2-0-2
B 436	436	0	12-2-13
G 436	436	0	2-0-2

UNFACTORED REACTIONS

1ST CASE	SNOW	LIVE	PERMALIVE	WIND	DEAD	SOIL
JT COMBINED	F	B	G			
264	174/0	0/0	0/0	0/0	90/0	0/0
286	192/0	0/0	0/0	0/0	94/0	0/0
316	166/0	0/0	0/0	0/0	149/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINTS(F, B, G)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E, F, C-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

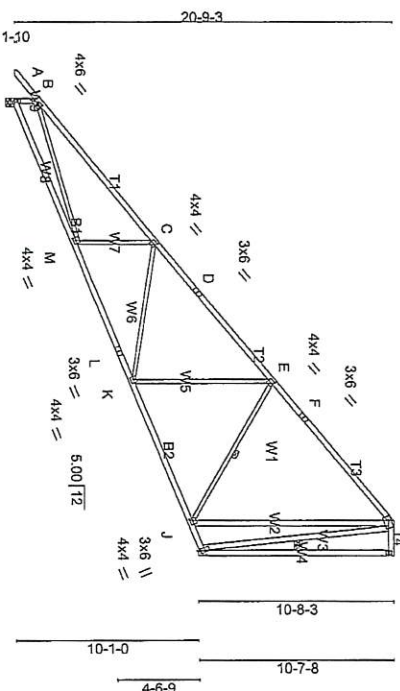
CHORDS	FACTORED	VERT. LOAD	CS1 (L.C)	MAX. UNBRACED	MEMB.	MAX. FACTORED	MAX. CS1 (L.C)
MEMB.	FORCE (LBS)	FROM	TO	FR-TO	FR-TO	FORCE (LBS)	MAX (L.C)
A-B	0/11	-78.0	-78.0	0.01 (1)	10.00	-270/0	0.19 (1)
B-1	-77/31	-78.0	-78.0	0.10 (1)	6.25	-211/0	0.11 (1)
I-C	-179/0	-78.0	-78.0	0.47 (1)	6.25	-487/0	0.00 (1)
C-D	-39/0	-78.0	-78.0	0.46 (1)	6.25		
D-E	-39/0	-78.0	-78.0	0.46 (1)	6.25		
F-E	-185/0	0.0	0.0	0.16 (1)	6.25		
B-H	0/149	-18.5	-18.5	0.16 (1)	10.00		
H-G	0/149	-18.5	-18.5	0.21 (4)	10.00		
G-F	0/149	-18.5	-18.5	0.21 (4)	10.00		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



A-18023490

ID:MF8titaTok7Dlbp14B71Xys4PG-icn1JugPFQhVudx7AOKS3hCRW47ZpSGJShozKE7h
 Version 8,200 S Jan 6 2018 MITek Industries, Inc. Sat Feb 17 13:18:52 2018 Page 1
 1-6-8-0-0-6-0 7-4-8 7-10-8 7-8-0 15-6-9 7-8-0 23-2-9 24-8-0 25-1-8
 1-6-8-0-0 7-4-8 7-10-8 7-8-0 10.00 [12] 5x8 = 2x4 II
 1-5-7-5-8
 Scale = 1:120.9

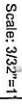


24-4-8
 6-8
 0-0 7-10-8 7-10-8 7-8-0 15-6-9 7-8-0 23-2-9 24-8-0 25-1-8
 6-8
 1-5-16 5-1
 2x4 II
 4x6
 4x4
 3x6
 4x4
 3x6
 4x4
 5.00 [12]
 4x4
 3x6
 4x4
 10-8-3
 10-7-8
 4-6-9
 10-1-0
 10-8-3
 10-7-8
 4-6-9

TOTAL WEIGHT = 136 lb [M/F]

LUMBER				N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR	DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER			
				BEARINGS			
A - D	2x4	DRY	SPF	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	RECORD
E - F	2x4	DRY	SPF	VERT HORIZ	VERT HORIZ	BRG PLATE	BRG PLATE
G - H	2x4	DRY	SPF	1	1212	0	HANGER BY OTHERS
I - J	2x4	DRY	SPF	N	1330	0	MIN. SEAT SIZE: 1-8
K - L	2x4	DRY	SPF			5-8	1-13
				DESIGN CRITERIA			
				SPECIFIED LOADS:			
				TOP CH.	DL =	210	PSF
				BOT CH.	DL =	60	PSF
				DL =	0.0	PSF	
				TOTAL LOAD =	7.4	PSF	
					34.4	PSF	
				SPACING = 24.0 IN. C/C			

16-8-2 7-11-13 24-8-0-25-1-8
 8-2-13 5-8
 8-5-5
 7-11-5
 1-6-8-0-0-6-0
 3-6-8-6-0

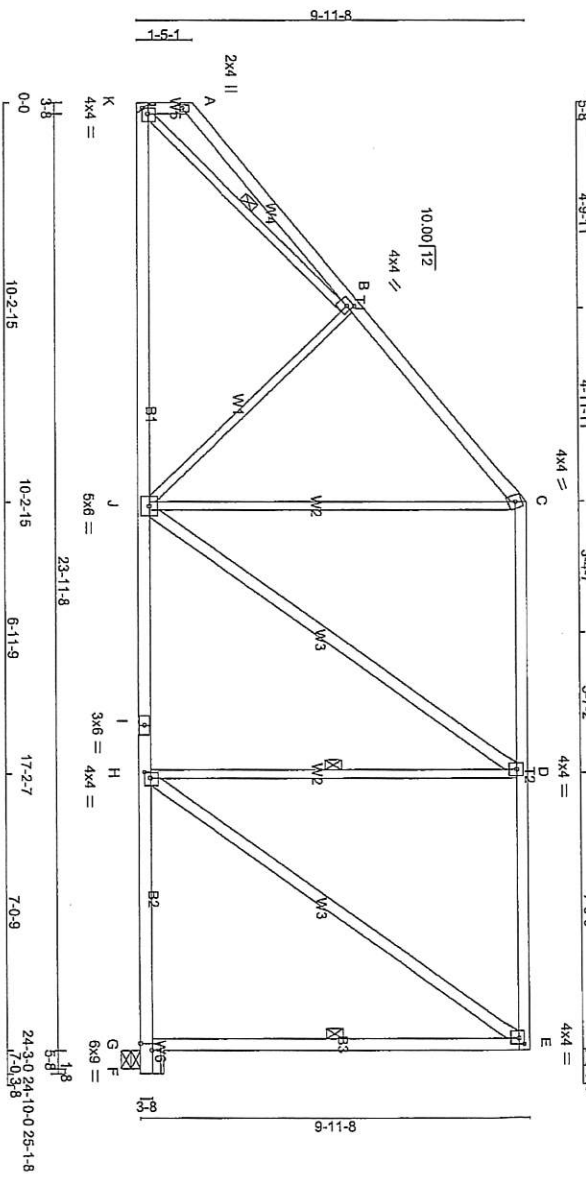
TOTAL WEIGHT = $7 \times 138 = 969 \text{ lb}$

DESIGN CRITERIA

JSI METAL=0.62 (K) (INPUT = 1.00)

JSI METAL=0.62 (K) (INPUT = 1.00)





TOTAL WEIGHT = 7 X 125 = 877 lb

LUMBER		N.L.G.A. RULES		CHORDS		LUMBER		DESCR.	
JT	TYPE	SIZE	DRY	No.2	DRY	No.2	DRY	No.2	DRY
A	TMVW-1	MT20	2.0	4.0	1.75	1.50			
B	TMVW-1	MT20	4.0	4.0	1.75	1.50			
C	TMVW-1	MT20	4.0	4.0	1.75	1.50			
D	TMVW-1	MT20	4.0	4.0	1.75	1.50			
E	TMVW-1	MT20	4.0	4.0	1.75	1.50			
F	BMVW-1	MT20	6.0	8.0	Edge 2.00				
G	BMVW-1	MT20	4.0	4.0	1.75	1.75			
H	BS-1	MT20	3.0	6.0					
I	BS-1	MT20	5.0	6.0					
J	BMVW-1	MT20	4.0	4.0	1.75	1.75			
K	BMVW-1	MT20	4.0	4.0	1.75	1.75			
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.									
ALL WEBS									
J	- D	2x4	DRY	No.2	SPF				
H	- E	2x4	DRY	No.2	SPF				
DRY - SEASONED LUMBER.									
UNFACTORED REACTIONS									
JT	COMBINED	506 / 0	LIVE	0 / 0	PERMALIVE	0 / 0	DEAD	347 / 0	SOIL
K	838	537 / 0	0 / 0	0 / 0	0 / 0	0 / 0	402 / 0	0 / 0	0 / 0
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINTS G									
BRACING									
FOR SECTION C-E, MAX. PURLIN SPACING = 2.00 FT.									
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.71 FT.									
MAX. UNBRACED BOTTOM CHORD LENGTH = 5.87 FT. OR RIGID CEILING DIRECTLY APPLIED.									
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.									
1 LATERAL BRACE(S) AT 1/2 LENGTH OF B-K, D-H.									
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW									
LOADING									
TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
MEMB.	FACTORED FORCE (LBS)	VERT. LOAD C1	MAX. FACTORED VERT. LOAD C1	MAX. FACTORED VERT. LOAD C1	MEMB.	FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD C1	MAX. FACTORED VERT. LOAD C1	MAX. FACTORED VERT. LOAD C1
FR-TO	0.0	0.0	0.02 (1)	7.81	J-C	0.256	0.06 (4)	0.06 (4)	0.06 (4)
K-A	-153 / 0	-78.0	0.32 (1)	10.00	K-B	-1330 / 0	0.48 (1)	0.48 (1)	0.48 (1)
B-A	-78.0	-78.0	0.38 (1)	5.71	B-J	-234 / 0	0.28 (1)	0.28 (1)	0.28 (1)
B-C	-1018 / 0	-46.5	0.29 (1)	2.00	J-D	0.128	0.05 (1)	0.05 (1)	0.05 (1)
D-E	-738 / 0	-85.5	0.74 (1)	2.00	H-D	-840 / 0	0.56 (1)	0.56 (1)	0.56 (1)
K-J	-685 / 0	-18.5	0.50 (4)	10.00	H-E	0.1180	0.19 (1)	0.19 (1)	0.19 (1)
J-I	0 / 919	-18.5	0.49 (4)	10.00					
I-H	0 / 685	-18.5	0.49 (4)	10.00					
H-G	0 / 0	-18.5	0.19 (4)	10.00					
G-F	-1182 / 0	0.0	0.07 (1)	5.87					

PLATES (table is in inches)									
JT	TYPE	PLATES	W	LEN	Y	X			
A	TMVW-1	MT20	2.0	4.0	4.0				
B	TMVW-1	MT20	4.0	4.0	1.75	1.50			
C	TTW-m	MT20	4.0	4.0	4.0				
D	TMVW-1	MT20	4.0	4.0	4.0				
E	TMVW-1	MT20	4.0	4.0	1.75	1.75			
F	BMVW-1	MT20	6.0	8.0	Edge 2.00				
G	BMVW-1	MT20	4.0	4.0	1.75	1.75			
H	BS-1	MT20	3.0	6.0					
I	BS-1	MT20	5.0	6.0					
J	BMVW-1	MT20	4.0	4.0	1.75	1.75			
K	BMVW-1	MT20	4.0	4.0	1.75	1.75			
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.									
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.71 FT.									
MAX. UNBRACED BOTTOM CHORD LENGTH = 5.87 FT. OR RIGID CEILING DIRECTLY APPLIED.									
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.									
1 LATERAL BRACE(S) AT 1/2 LENGTH OF B-K, D-H.									
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW									
LOADING									
TOTAL LOAD CASES: (4)									
PART 9, NBCC 2010, NBCC 2015									
THIS DESIGN COMPLIES WITH:									
- PART 9 OF NBC 2012, OSC 2018									
- CSA 086-09, CSA 086-14									
- TPIC 2011, TPIC 2014									
(55 % OF 23.0 P.S.F., G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F., SPECIFIED ROOF LIVE LOAD									
ALLOWABLE DEF'L (L) = U 860 (0.80")									
CALCULATED VERT. DEF'L (L) = U 999 (0.03")									
ALLOWABLE DEF'L (TL) = U 860 (0.80")									
CALCULATED VERT. DEF'L (TL) = U 860 (0.34")									

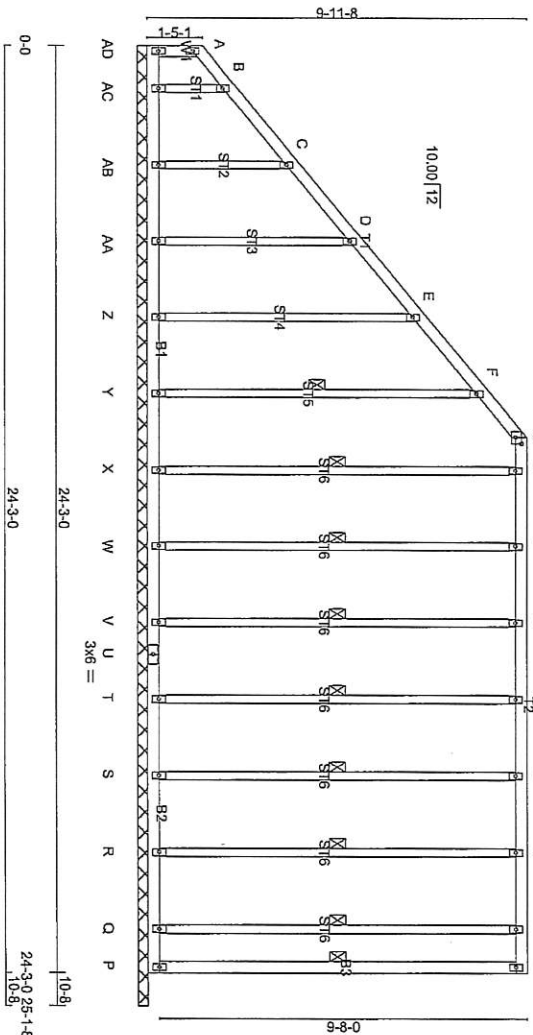
NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



A-18023496

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292593	H10G	1	1	TRUSS DESC.	
Alpha Roof Truss, Maple					

0-0-5-8
5-8
13-1-14
3x4 =
13-7-6
10-7-10
Scale = 1:57.7



TOTAL WEIGHT = 145 lb

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINTS)

ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	BRACING FOR SECTION G-O, MAX. PURLIN SPACING = 2.00 FT. FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
ALL GABLE WEBS	2x3	DRY	No.2	SPF	MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
DRY: SEASONED LUMBER.					

LUMBER	N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
AD - A	2x4	DRY	No.2	SPF	
AG - G	2x4	DRY	No.2	SPF	
AO - O	2x4	DRY	No.2	SPF	
AD - U	2x4	DRY	No.2	SPF	
U - P	2x4	DRY	No.2	SPF	
P - O	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
ALL GABLE WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					
GABLE STUDS SPACED AT 24-0 OC.					

1 LATERAL BRACE(S) AT 1/2 LENGTH OF O-P, N-Q, M-R, L-S, K-T, J-V, H-X, F-Y.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

CHORDS	MEMB.	FACTORED FORCE (LBS)	VERT. LOAD (LBS)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (I/C)
AD-A	-48.0	0.0	0.0	7.81	Q-N	-128.0	0.09 (1)
A-B	-17.0	-78.0	0.02 (1)	6.25	R-M	-180.0	0.12 (1)
B-C	-12.0	-78.0	0.04 (1)	6.25	S-L	-170.0	0.11 (1)
C-D	-10.0	-78.0	0.04 (1)	6.25	T-K	-171.0	0.11 (1)
D-E	-6.0	-78.0	0.04 (1)	10.00	W-J	-171.0	0.11 (1)
E-F	-3.0	-78.0	0.04 (1)	10.00	X-H	-173.0	0.11 (1)
F-G	-23.0	-78.0	0.04 (1)	6.25	Y-F	-169.0	0.08 (1)
G-H	0.0	-85.5	0.04 (1)	2.00	Z-E	-164.0	0.15 (1)
H-I	0.0	-85.5	0.04 (1)	2.00	AA-D	-153.0	0.08 (1)
I-J	0.0	-85.5	0.04 (1)	2.00	AB-C	-158.0	0.04 (1)
J-K	0.0	-85.5	0.04 (1)	2.00	AC-B	-117.0	0.02 (1)
K-L	0.0	-85.5	0.04 (1)	2.00			
L-M	0.0	-85.5	0.04 (1)	2.00			
M-N	0.0	-85.5	0.04 (1)	2.00			
N-O	0.0	-85.5	0.02 (1)	2.00			
AD-AC	0.0	-18.5	0.01 (1)	10.00			
AC-AB	0.0	-18.5	0.02 (1)	10.00			
AB-AA	0.0	-18.5	0.02 (1)	10.00			
AA-Z	0.0	-18.5	0.01 (1)	10.00			
Z-Y	0.0	-18.5	0.01 (1)	10.00			
Y-X	0.0	-18.5	0.01 (1)	10.00			
X-W	0.0	-18.5	0.01 (1)	10.00			
W-V	0.0	-18.5	0.01 (1)	10.00			
V-U	0.0	-18.5	0.01 (1)	10.00			
U-T	0.0	-18.5	0.01 (1)	10.00			
T-S	0.0	-18.5	0.01 (1)	10.00			
S-R	0.0	-18.5	0.02 (1)	10.00			
R-Q	0.0	-18.5	0.02 (1)	10.00			
Q-P	0.0	-18.5	0.01 (1)	10.00			
P-O	-33.0	0.0	0.0	6.25			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
TOP CH. LL = 6.0 PSF
BOT CH. LL = 0.0 PSF
BOT CH. LL = 7.4 PSF
TOTAL LOAD = 34.4 PSF
SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND 6.00/12 AND RESPECTIVE WALL HEIGHTS OF 6.0 AND 6.0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015
-PART 9 OF CBC 2012, OBC 2018
-CSA 086-09, CSA 086-14
-TPIC 2011, TPIC 2014

(55% OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CS1: TC=0.04/1.00 (L-M-1), BC=0.02/1.00 (AB-AC-4), WB=0.15/1.00 (E-Z-1), SS=0.07/1.00 (M-N-1)

DOL LUMBER=1.00 NAL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(PV) SHEAR SECTION (PSI) (PL) (PL) MAX MIN MAX MIN MAX MIN
MIT20 618 354 1667 788 1987 1656
PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 6.0 Deg.
JIS GRIP=0.84 (3) (INPUT = 0.90)
JIS METAL=0.08 (7) (INPUT = 1.00)

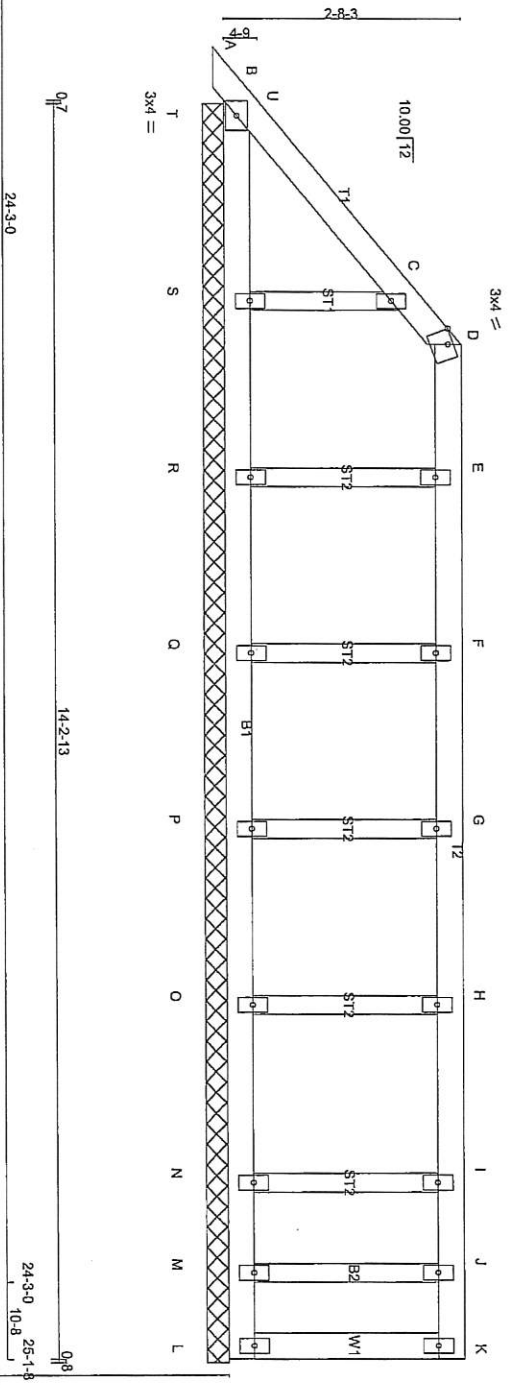
NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



A-18023497

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292593	H11P	1	1	TRUSS DESC.	
Alpha Roof Truss, Maple					

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ID:MFBtrn1ok7Dlpj14B71Xys4PG-BndPVw6a87YL0r7INytm1wepOe7a8s_X1wzKzERY
24-3-0 24-8-0 25-1-8
5-0-1 5-8
Scale = 1/25.0



24-3-0

14-2-13

24-3-0 25-1-8

TOTAL WEIGHT = 48 lb

LUMBER	N.L.G.A. RULES	DESCR.	DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER	DESIGN CRITERIA
CHORDS	SIZE	LUMBER	BEARINGS	SPECIFIED LOADS:
A - D	2x4	DRY	THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.	TOP CH. LL = 24.0 PSF
D - K	2x4	DRY	THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.	DL = 6.0 PSF
L - K	2x3	DRY	BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)	BOT CH. LL = 7.4 PSF
M - J	2x4	DRY		TOTAL LOAD = 34.4 PSF
B - L	2x4	DRY		SPACING = 24.0 IN. C/C
ALL WEBS	2x3	DRY		
ALL GABLE WEBS	2x3	DRY		
DRY SEASONED LUMBER.				
GABLE STUDS SPACED AT 24-0 OC.				

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

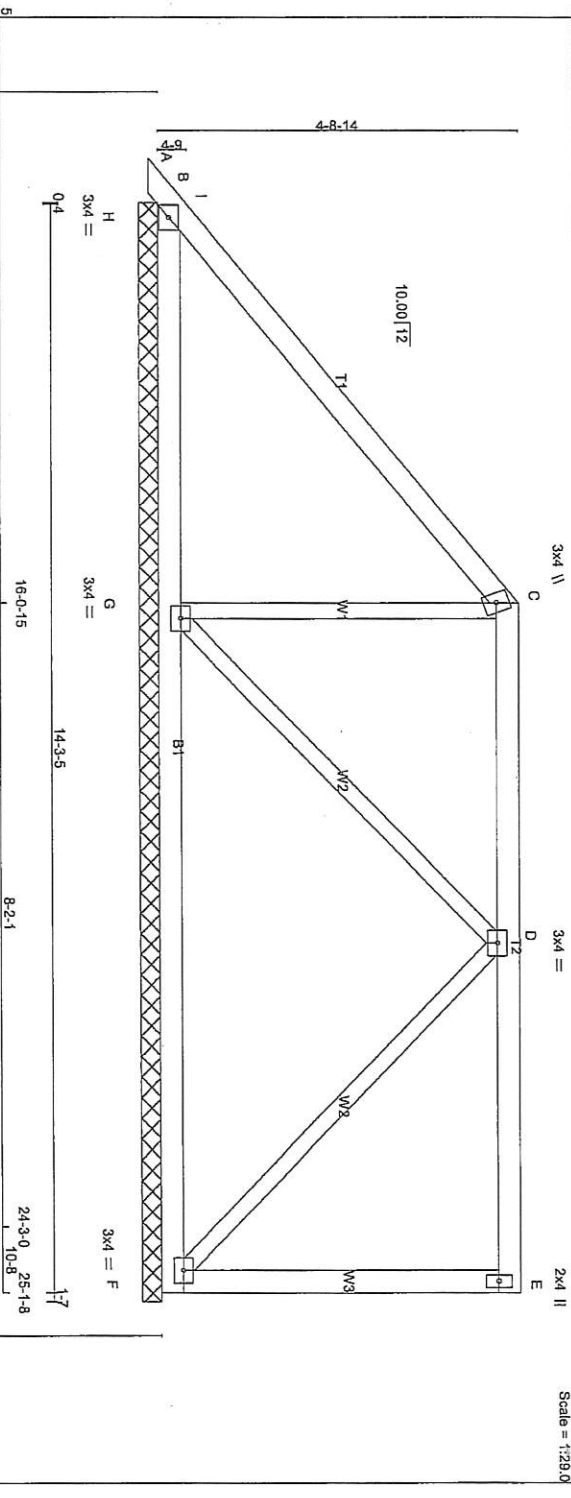
LOADING
TOTAL LOAD CASES: (4)

PLATES (table is in inches)	W	LEN	Y	X	CHORDS	FACTORED	MAX.	WEBS	MAX.
JT TYPE					MEMB.	FORCE	VERT. LOAD LC1	MEMB.	FORCE
B TM81-H	MT20	3.0	4.0		FR-TO	0.12	-78.0	FR-TO	-128.0
C, E, F, G, H, I	MT20	2.0	4.0	Edge	A-B	-12.0	-78.0	O-H	-161.0
D TMV+H	MT20	2.0	4.0		B-U	-78.0	-78.0	O-H	-155.0
J TMV+P	MT20	2.0	4.0		U-C	0.1	-78.0	P-G	0.03 (1)
K TMV+P	MT20	2.0	4.0		C-D	-42.0	-78.0	Q-F	0.03 (1)
L BMV1+P	MT20	2.0	4.0		D-E	0.0	-78.0	R-E	-155.0
M BMV1+P	MT20	2.0	4.0		E-F	0.0	-78.0	S-C	-173.0
N, O, P, Q, R, S	MT20	2.0	4.0		F-G	0.0	-78.0	T-U	-65.0
N BMV1+H	MT20	2.0	4.0		G-H	0.0	-78.0		
					H-I	0.0	-78.0		
					I-J	0.0	-78.0		
					J-K	0.0	-78.0		
					L-K	-33.0	0.0		
					M-J	-71.0	0.0		
					B-T	0.9	-18.5		
					T-S	0.9	-18.5		
					S-R	0.0	-18.5		
					R-Q	0.0	-18.5		
					Q-P	0.0	-18.5		
					P-O	0.0	-18.5		
					O-N	0.0	-18.5		
					N-M	0.0	-18.5		
					M-L	0.0	-18.5		

NOTE: Lateral brace(s) shown shall be 1x4
for Part 9 design as per OBC 9.23.13.11,
and no less than 2x4 for Part 4 design



A-18023498



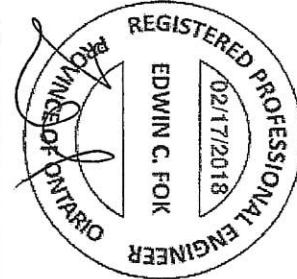
LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER			
N.L.G.A	CHORDS	SIZE	LUMBER	DESCR	FACTORED	MAXIMUM FACTORED	INPUT
A - C	2x4	DRY	No.2	SPF	GROSS REACTION	GROSS REACTION	BRG IN-SX
C - E	2x4	DRY	No.2	SPF	VERT	HORIZ	DOWN
F - F	2x4	DRY	No.2	SPF	VERT	HORIZ	UP/LIFT
B - F	2x4	DRY	No.2	SPF	G	669	0
ALL WEBS	2x3	DRY	No.2	SPF	G	669	0
DRY; SEASONED LUMBER.							

PLATES				UNFACTORED REACTIONS			
JT TYPE	PLATES	W	LEN	Y	X	IST LOOSE	MAX MIN. COMPONENT REACTIONS
B	TM8-1	3.0	4.0			COMBINED	SNOW
C	TM8-1	3.0	4.0			F	311
D	TM8-1	3.0	4.0			B	217
E	TM8-1	3.0	4.0			G	482
F	TM8-1	3.0	4.0				
G	TM8-1	3.0	4.0				
H	TM8-1	3.0	4.0				
I	TM8-1	3.0	4.0				
J	TM8-1	3.0	4.0				
K	TM8-1	3.0	4.0				
L	TM8-1	3.0	4.0				
M	TM8-1	3.0	4.0				
N	TM8-1	3.0	4.0				
O	TM8-1	3.0	4.0				
P	TM8-1	3.0	4.0				
Q	TM8-1	3.0	4.0				
R	TM8-1	3.0	4.0				
S	TM8-1	3.0	4.0				
T	TM8-1	3.0	4.0				
U	TM8-1	3.0	4.0				
V	TM8-1	3.0	4.0				
W	TM8-1	3.0	4.0				
X	TM8-1	3.0	4.0				
Y	TM8-1	3.0	4.0				
Z	TM8-1	3.0	4.0				

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, G.
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

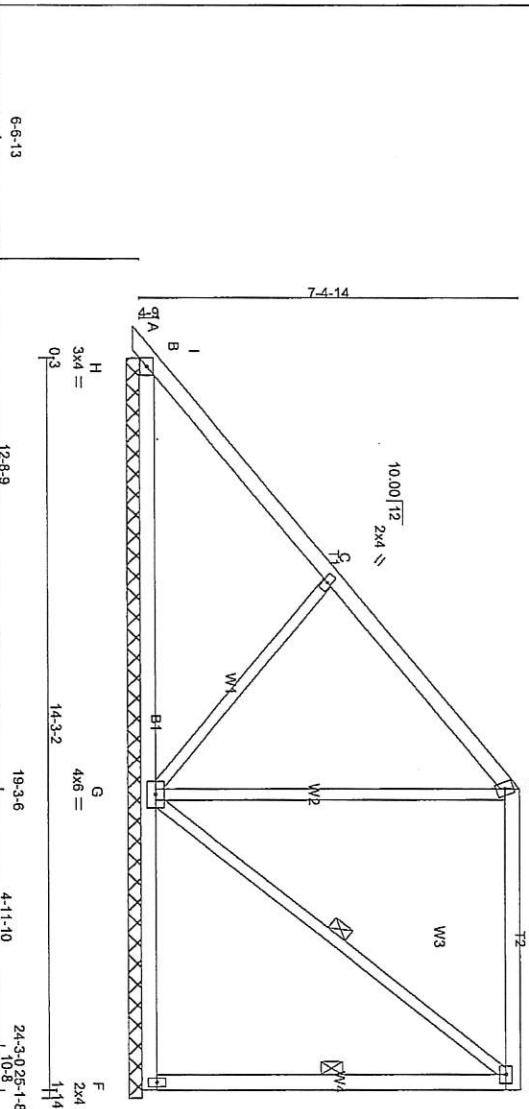
LOADING				TOTAL LOAD CASES: (4)			
CHORDS	MEMB.	FACTORED	VERT. LOAD	FACTORED	VERT. LOAD	FACTORED	VERT. LOAD
FR-TO	0/12	-78.0	0.02 (1)	10.00	G-C	-254/0	0.08 (1)
A-B	0/229	-78.0	0.19 (1)	10.00	G-D	-279/0	0.20 (1)
I-C	-89/0	-78.0	0.26 (1)	6.25	D-F	-344/0	0.24 (1)
C-D	-51/0	-78.0	0.27 (1)	6.25	H-I	-655/0	0.00 (1)
D-E	0/0	-78.0	0.27 (1)	10.00			
F-E	-135/0	0.0	0.05 (1)	7.81			
B-H	0/57	-18.5	0.24 (1)	10.00			
H-G	0/57	-18.5	0.35 (4)	10.00			
G-F	0/246	-18.5	0.36 (4)	10.00			

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292593	H13P	1	1	TRUSS DESC.	
Alpha Roof Truss, Maple					

6-6-13	8-6-1	15-2-14	4-0-8	19-3-6	4-11-10
ID: MfBiltraTolK7DlpT4B71Xys4Pq-Fa14I2L_cKR8bberUpfJ03w9NvNs10F14KJqIKyTZkERg					
Version 8.200 S Jan 6 2018 Mitek Industries, Inc. Sat Feb 17 13:21:07 2018 Page 1					
24-3-0 24-8-0 25-1-8					
F.05.5					
Scale = 1:3.0					



6-6-13	12-9-9	19-3-6	4-11-10	24-3-0 25-1-8	1-10-8
TOTAL WEIGHT = 61 lb					

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	SPF
D - E	2x4	DRY	No.2	SPF	SPF
F - E	2x4	DRY	No.2	SPF	SPF
B - F	2x4	DRY	No.2	SPF	SPF
ALL WEBS	2x3	DRY	No.2	SPF	SPF
DRY, SEASONED LUMBER.					

PLATES	(table is in inches)	W	LEN	Y	X
JT TYPE	PLATES	3.0	4.0		
B	TMB-1-1	3.0	4.0		
C	TMB-1-1	3.0	4.0		
D	TMB-1-1	3.0	4.0		
E	TMB-1-1	3.0	4.0		
F	TMB-1-1	3.0	4.0		
G	TMB-1-1	3.0	4.0		
H	TMB-1-1	3.0	4.0		
I	TMB-1-1	3.0	4.0		
J	TMB-1-1	3.0	4.0		
K	TMB-1-1	3.0	4.0		
L	TMB-1-1	3.0	4.0		
M	TMB-1-1	3.0	4.0		
N	TMB-1-1	3.0	4.0		
O	TMB-1-1	3.0	4.0		
P	TMB-1-1	3.0	4.0		
Q	TMB-1-1	3.0	4.0		
R	TMB-1-1	3.0	4.0		
S	TMB-1-1	3.0	4.0		
T	TMB-1-1	3.0	4.0		
U	TMB-1-1	3.0	4.0		
V	TMB-1-1	3.0	4.0		
W	TMB-1-1	3.0	4.0		
X	TMB-1-1	3.0	4.0		
Y	TMB-1-1	3.0	4.0		
Z	TMB-1-1	3.0	4.0		

UNFACTORED REACTIONS	1ST LOUSE	MAX MIN COMPONENT REACTIONS	WIND	DEAD	SOIL
JT	COMBINED	SNOW	LIVE	PERMALIVE	
F	158	98/0	0/0	0/0	0/0
B	295	186/0	0/0	0/0	109/0
G	557	336/0	0/0	0/0	220/0

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. CS1 (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO	0/12	-78.0	-78.0	0.02 (1)	10.00	G-D	-367/0	0.36 (1)	
A-B	-459/0	-78.0	-78.0	0.16 (4)	6.25	G-G	-326/0	0.17 (1)	
B-1	-253/0	-78.0	-78.0	0.25 (1)	6.25	G-E	-48/0	0.03 (1)	
C-D	-8/5	-78.0	-78.0	0.24 (1)	10.00	H-1	0/300	0.00 (1)	
D-E	0/30	-78.0	-78.0	0.46 (1)	10.00				
F-E	-190/0	0.0	0.0	0.05 (1)	6.25				
B-H	0/223	-18.5	-18.5	0.14 (4)	10.00				
H-G	0/223	-18.5	-18.5	0.28 (4)	10.00				
G-F	0/0	-18.5	-18.5	0.26 (4)	10.00				

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per CBC 9.2.3.13.11, and no less than 2x4 for Part 4 design

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GR50(DR) SHEAR SECTION

(PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

M720 618 354 1667 788 1967 1656

PLATE PLACEMENT TOL. = 0.250 inches

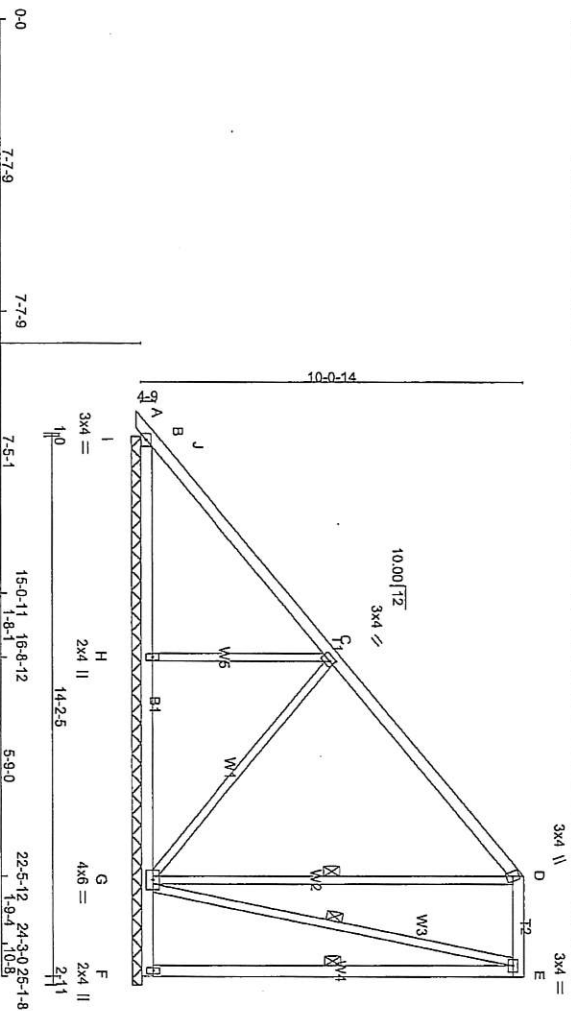
PLATE ROTATION TOL. = 5.0 Deg.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



A-18023500

	ID:Mf8litaToK/Dlp7tAB71Xs4Pg-N4J_NN/S8/y4IaxOgFZR5yBsp6xIdEIJmWdzERr
0-0-58	15-0-11 16-6-12
P-8	-1-8-1 -
7-2-1	5-9-0
	22-5-12 24-3-0 24-8-0 25-1-8
	-1-9-1 -5-0-5-8

[illegible]

PLATES (table is in inches)					UNFACTORED REACTIONS					SLOPE OF 2.00/12 MINIMUM				
JT	PLATES	W	LEN	Y	1ST LOOSE	MAX MIN.	COMPONENT REACTIONS	DEAD	SOIL	THIS TRUSS IS DESIGNED FOR RESIDENTIAL LOADS PER THE REQUIREMENTS OF PART 8, NBCQ 2010, NBCQ 2015				
JT TYPE				X	COMBINED	LIVE	PERALIVE	WIND						
B	TMW-1	3.0	4.0		64	0/0	0/0	0/0	0/0	THIS DESIGN COMPLIES WITH: - PART 8 OF OBC 2012, OBC 2018 - CSA 006-09, CSA 006-14				
C	TMW-4	3.0	4.0	1.50	168/0	0/0	0/0	86/0	0/0					
B	TMW-1	3.0	4.0		168/0	0/0	0/0	86/0	0/0	BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINTS) F, B, G, H				
C	TMW-4	3.0	4.0		168/0	0/0	0/0	86/0	0/0					
E	TMW-1	3.0	4.0		347	0/0	0/0	133/0	0/0					
F	TMW-4	3.0	4.0		214/0	0/0	0/0	133/0	0/0					
G	BMWV-1	4.0	6.0		194/0	0/0	0/0	148/0	0/0					
H	BMWV-4	4.0	6.0		194/0	0/0	0/0	148/0	0/0					

BRAING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1. LATERAL BRACES) AT 1/2 LENGTH OF E-F, D-G, E-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

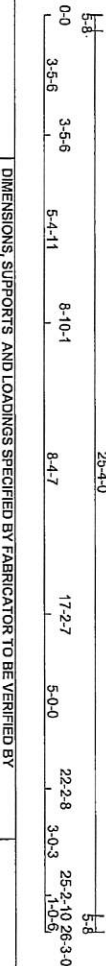
[illegible]

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



A-18023501

0.058	3.5-6	5.4-11	8-10-1	4.2-3	13-0-4	4.2-3	17-2-7	5-0-0	22-2-8	3-0-3	25-2-10	25-0-8	26-5-0	27-9-8
5-4	2-1-14										6-14-5	8-15-5		
		4x6 //			4x4 =		5x6 //							

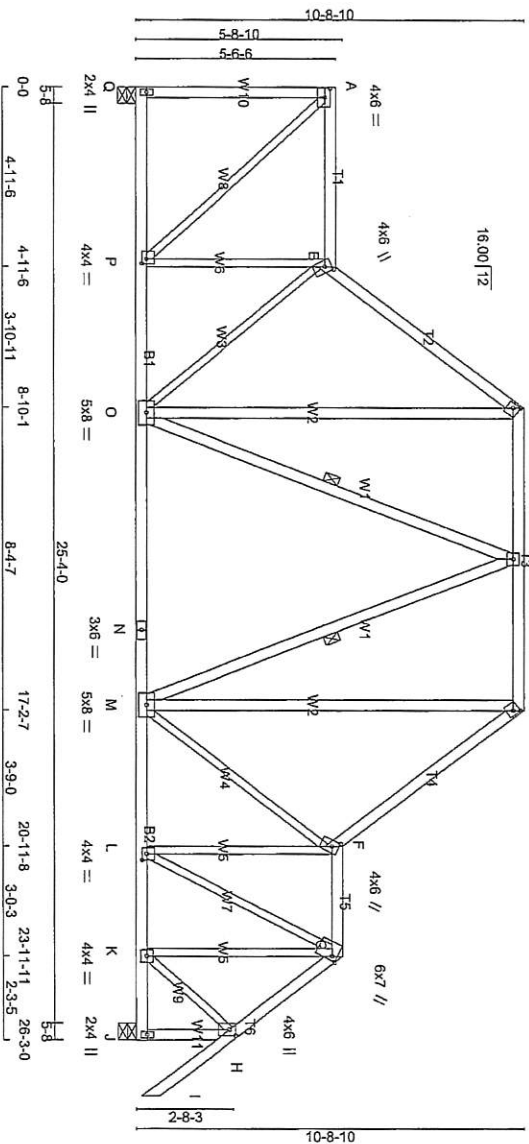


ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



A-18023503 CONTINUED ON PAGE 2

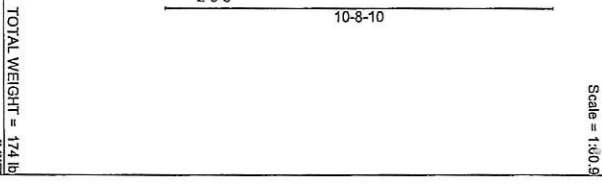


TOTAL WEIGHT = 163 lb [M/F]

LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER			
N.L.G.A. RULES	CHOIRS	SIZE	LUMBER	DESCR.	FACTORED	MAXIMUM FACTORED	INPUT
Q - A	2x4	DRY	No.2	SPF	VERT	GROSS REACTION	BRG
A - B	2x4	DRY	No.2	SPF	DOWN	HORIZ	IN-SX
B - C	2x4	DRY	No.2	SPF	UP	LIFT	IN-SX
C - E	2x4	DRY	No.2	SPF	Q	1294	0
E - F	2x4	DRY	No.2	SPF	Q	1431	0
F - G	2x4	DRY	No.2	SPF	Q	1431	0
G - I	2x4	DRY	No.2	SPF	Q	1431	0
I - H	2x4	DRY	No.2	SPF	Q	1431	0
H - J	2x4	DRY	No.2	SPF	Q	1431	0
J	2x4	DRY	No.2	SPF	Q	1431	0
ALL WEBS	2x3	DRY	No.2	SPF	Q	1431	0
EXCEPT	2x4	DRY	No.2	SPF	Q	1431	0
O - C	2x4	DRY	No.2	SPF	Q	1431	0
D - M	2x4	DRY	No.2	SPF	Q	1431	0
M - E	2x4	DRY	No.2	SPF	Q	1431	0
DRY, SEASONED LUMBER.							

PLATES (table is in inches)				UNFACTORED REACTIONS			
JT TYPE	PLATES	W	LEN	Y	X	1ST LOUSE	MAX MIN. COMPONENT REACTIONS
A	TTWV-1	4.0	6.0	1.75	3.00	Q	COMBINED
B	TTWV-1	4.0	6.0	2.25	2.00	J	925
C	TTWV-1	4.0	6.0	2.25	2.00	J	1020
D	TTWV-1	4.0	6.0	2.25	2.00	J	1020
E	TTWV-1	4.0	6.0	2.25	2.00	J	1020
F	TTWV-1	4.0	6.0	2.25	2.00	J	1020
G	TTWV-1	4.0	6.0	2.25	2.00	J	1020
H	TTWV-1	4.0	6.0	2.25	2.00	J	1020
I	TTWV-1	4.0	6.0	2.25	2.00	J	1020
J	TTWV-1	4.0	6.0	2.25	2.00	J	1020
K	TTWV-1	4.0	6.0	2.25	2.00	J	1020
L	TTWV-1	4.0	6.0	2.25	2.00	J	1020
M	TTWV-1	4.0	6.0	2.25	2.00	J	1020
N	TTWV-1	4.0	6.0	2.25	2.00	J	1020
O	TTWV-1	4.0	6.0	2.25	2.00	J	1020
P	TTWV-1	4.0	6.0	2.25	2.00	J	1020
Q	TTWV-1	4.0	6.0	2.25	2.00	J	1020
EDGE - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.							

NOTE: Lateral braces shown shall be 1x4 and no less than 2x4 for Part 4 design									
G-H	-878/0	-78.0	-78.0	0.12 (1)	6.25	M-F	-443/0	0.35 (1)	DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
H-I	0/52	-78.0	-78.0	0.15 (1)	5.00	L-G	-899/0	0.45 (1)	COMP=1.10 SHEAR=1.10 TENS=1.10
J-H	-1421/0	0.0	0.0	0.21 (1)	6.84	K-G	0/1045	0.24 (1)	
						K-H	-403/0	0.20 (1)	
							0/692	0.16 (1)	
COMPANION LIVE LOAD FACTOR= 1.00									
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.									
NAIL VALUES									

TOTAL WEIGHT = 174 lb88 1987 1656

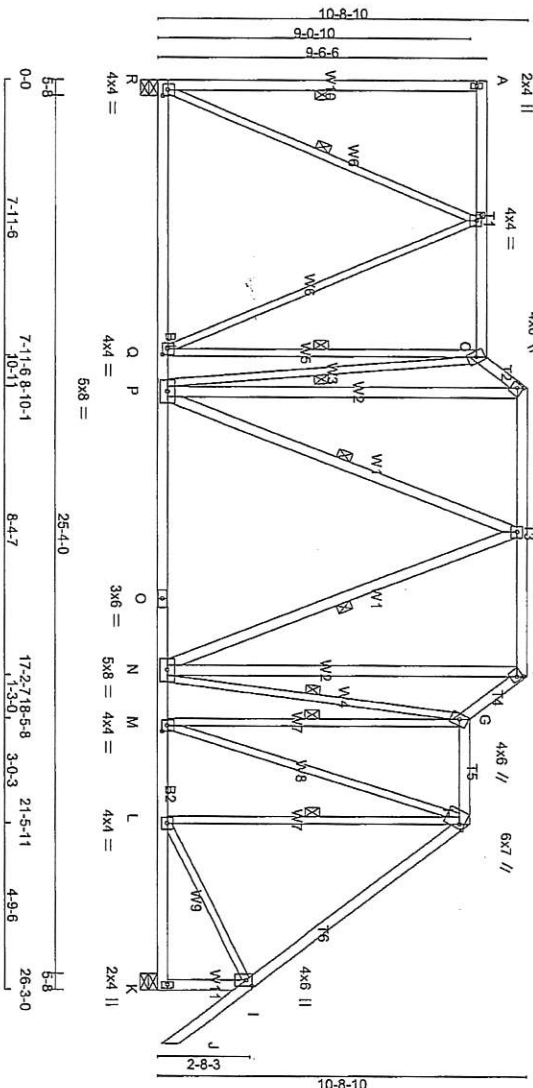
URER IS NOT
ITY CONTROL IN
RING PLANT.



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
292593	H19	1	1			
Alpa Roof Truss, Maple						

Version 8.2005 Jan 6 2018 MITKX Industries, Inc. Sat Feb 17 13:22:15 2018 Page 1
 ID:MFllm1a7ok7Dlbt14B71Xys4FG-F8mW7ebD7UJbeV0JXJqG22GRfBldkY7wLXzKEQC
 5-8 3-6-11 4-0-3 3-11-3 7-11-6 8-10-1 4-2-3 13-0-4 4-2-3 17-2-7 18-5-8 3-0-3 21-5-11 4-3-14 25-9-3 26-3-0 27-9-8
 5-8 1-6-8

Scale: 3/16"=1'



0-0	5-8	7-11-6	7-11-6 8-10-1	8-4-7	17-2-7 18-5-8	3-0-3	21-5-11	4-9-6	26-3-0
			10-11		1-3-0				

TOTAL WEIGHT = 193 lb

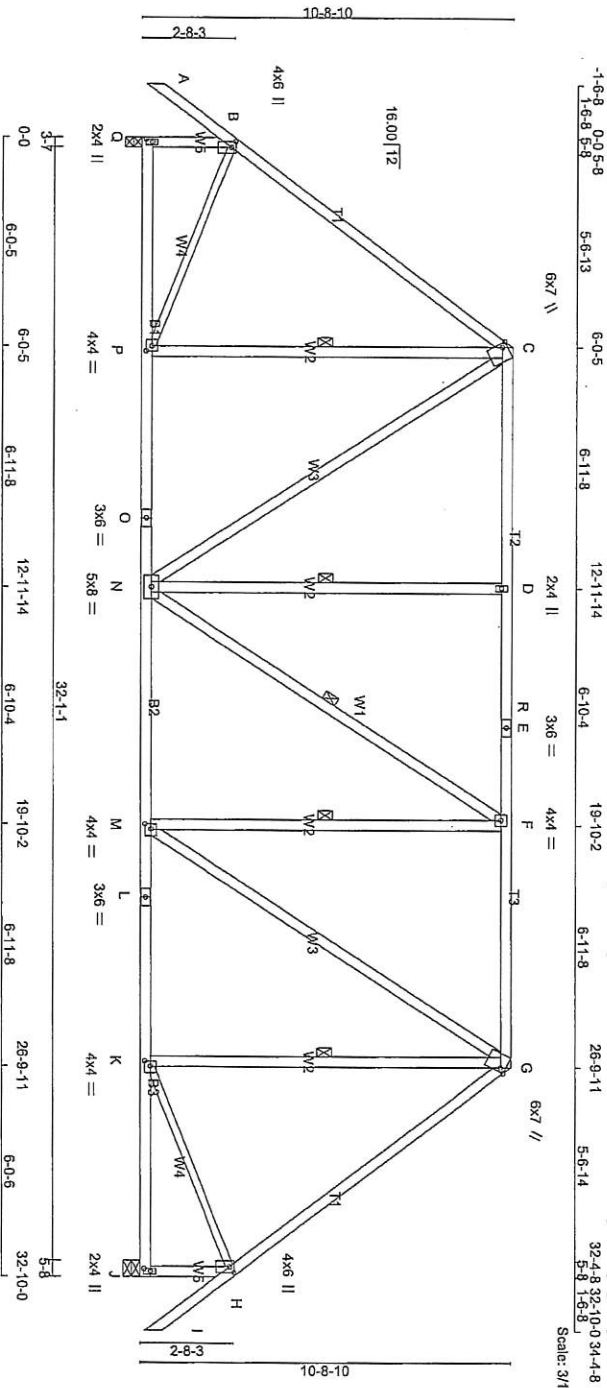
LUMBER	N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.	DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER	DESIGN CRITERIA
R - A	2x4	DRY	No.2	SPF	FACTORED	MAXIMUM FACTORED	SPECIFIED LOADS:
A - C	2x4	DRY	No.2	SPF	GROSS REACTION	BRG	TOP CH. LL = 21.0 PSF
C - D	2x4	DRY	No.2	SPF	DOWN	IN-SX	DL = 6.0 PSF
D - F	2x4	DRY	No.2	SPF	UP	IN-SX	BOT CH. LL = 0.0 PSF
F - G	2x4	DRY	No.2	SPF	DOWN	IN-SX	DL = 7.4 PSF
G - H	2x4	DRY	No.2	SPF	UP	IN-SX	TOTAL LOAD = 34.4 PSF
H - I	2x4	DRY	No.2	SPF	DOWN	IN-SX	
I - J	2x4	DRY	No.2	SPF	UP	IN-SX	
J - K	2x4	DRY	No.2	SPF	DOWN	IN-SX	
K - L	2x4	DRY	No.2	SPF	UP	IN-SX	
L - M	2x4	DRY	No.2	SPF	DOWN	IN-SX	
M - N	2x4	DRY	No.2	SPF	UP	IN-SX	
N - O	2x4	DRY	No.2	SPF	DOWN	IN-SX	
O - P	2x4	DRY	No.2	SPF	UP	IN-SX	
P - Q	2x4	DRY	No.2	SPF	DOWN	IN-SX	
Q - R	2x4	DRY	No.2	SPF	UP	IN-SX	
R - S	2x4	DRY	No.2	SPF	DOWN	IN-SX	
S - T	2x4	DRY	No.2	SPF	UP	IN-SX	
T - U	2x4	DRY	No.2	SPF	DOWN	IN-SX	
U - V	2x4	DRY	No.2	SPF	UP	IN-SX	
V - W	2x4	DRY	No.2	SPF	DOWN	IN-SX	
W - X	2x4	DRY	No.2	SPF	UP	IN-SX	
X - Y	2x4	DRY	No.2	SPF	DOWN	IN-SX	
Y - Z	2x4	DRY	No.2	SPF	UP	IN-SX	
Z - A	2x4	DRY	No.2	SPF	DOWN	IN-SX	

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE	2.0	4.0	1.75	2.00
A - B	2.0	4.0	1.75	2.00
B - C	2.0	4.0	1.75	2.00
C - D	2.0	4.0	1.75	2.00
D - E	2.0	4.0	1.75	2.00
E - F	2.0	4.0	1.75	2.00
F - G	2.0	4.0	1.75	2.00
G - H	2.0	4.0	1.75	2.00
H - I	2.0	4.0	1.75	2.00
I - J	2.0	4.0	1.75	2.00
J - K	2.0	4.0	1.75	2.00
K - L	2.0	4.0	1.75	2.00
L - M	2.0	4.0	1.75	2.00
M - N	2.0	4.0	1.75	2.00
N - O	2.0	4.0	1.75	2.00
O - P	2.0	4.0	1.75	2.00
P - Q	2.0	4.0	1.75	2.00
Q - R	2.0	4.0	1.75	2.00
R - S	2.0	4.0	1.75	2.00
S - T	2.0	4.0	1.75	2.00
T - U	2.0	4.0	1.75	2.00
U - V	2.0	4.0	1.75	2.00
V - W	2.0	4.0	1.75	2.00
W - X	2.0	4.0	1.75	2.00
X - Y	2.0	4.0	1.75	2.00
Y - Z	2.0	4.0	1.75	2.00
Z - A	2.0	4.0	1.75	2.00

CHORDS	FACTORED	VERT. LOAD	LC1	MAX	UNBRAC	MEMB.	W E B S	MAX. FACTORED	MAX	CS1 (I/C)
R - A	-18.0	0.0	0.0	0.05 (1)	6.25	Q - C	-619.0	0.37 (1)	0.37 (1)	
A - B	-78.0	0.0	0.0	0.05 (1)	6.25	P - D	-532.0	0.33 (1)	0.33 (1)	
B - C	-78.0	0.0	0.0	0.05 (1)	6.25	P - E	-238.0	0.16 (1)	0.16 (1)	
C - D	-78.0	0.0	0.0	0.05 (1)	6.25	E - N	-215.0	0.15 (1)	0.15 (1)	
D - E	-78.0	0.0	0.0	0.05 (1)	6.25	N - F	-446.0	0.24 (1)	0.24 (1)	
E - F	-78.0	0.0	0.0	0.05 (1)	6.25	M - G	-728.0	0.39 (1)	0.39 (1)	
F - G	-78.0	0.0	0.0	0.05 (1)	6.25	M - H	-169.0	0.09 (1)	0.09 (1)	
G - H	-78.0	0.0	0.0	0.05 (1)	6.25	L - I	-169.0	0.15 (1)	0.15 (1)	
H - I	-78.0	0.0	0.0	0.05 (1)	6.25	L - J	-1223.0	0.89 (1)	0.89 (1)	
I - J	-78.0	0.0	0.0	0.05 (1)	6.25	R - B	-1223.0	0.89 (1)	0.89 (1)	
J - K	-78.0	0.0	0.0	0.05 (1)	6.25	B - Q	-1223.0	0.89 (1)	0.89 (1)	
K - L	-78.0	0.0	0.0	0.05 (1)	6.25					
L - M	-78.0	0.0	0.0	0.05 (1)	6.25					
M - N	-78.0	0.0	0.0	0.05 (1)	6.25					
N - O	-78.0	0.0	0.0	0.05 (1)	6.25					
O - P	-78.0	0.0	0.0	0.05 (1)	6.25					
P - Q	-78.0	0.0	0.0	0.05 (1)	6.25					
Q - R	-78.0	0.0	0.0	0.05 (1)	6.25					
R - S	-78.0	0.0	0.0	0.05 (1)	6.25					
S - T	-78.0	0.0	0.0	0.05 (1)	6.25					
T - U	-78.0	0.0	0.0	0.05 (1)	6.25					
U - V	-78.0	0.0	0.0	0.05 (1)	6.25					
V - W	-78.0	0.0	0.0	0.05 (1)	6.25					
W - X	-78.0	0.0	0.0	0.05 (1)	6.25					
X - Y	-78.0	0.0	0.0	0.05 (1)	6.25					
Y - Z	-78.0	0.0	0.0	0.05 (1)	6.25					
Z - A	-78.0	0.0	0.0	0.05 (1)	6.25					

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.2.3, 13.1.1, and no less than 2x4 for Part 4 design





TOTAL WEIGHT = 6 X 194 = 1162 lb [M/F]

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		MAXIMUM FACTORED		INPUT		RECORD	
FACTORED	GROSS REACTION	MAXIMUM FACTORED	GROSS REACTION	BRG	BRG	RECORD	BRG
VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX
J	1781	0	1781	0	3.7	2.5	2.5
J	1781	0	1781	0	5.8	2.10	2.10

UNFACTORED REACTIONS

1ST CASE	MAX MAIN COMPONENT REACTIONS	WIND	DEAD	SOIL
COMBINED	SNOW	LIVE	PERMALIVE	
JT	1281	759 / 0	0 / 0	0 / 0
J	1281	759 / 0	0 / 0	522 / 0
J	1281	759 / 0	0 / 0	522 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING
FOR SECTION C.G. MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-P, D-N, F-M, G-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	FACTORED	MEMB.	FACTORED
FORCE (LBS)	VERT. LOAD (LBS)	FORCE (LBS)	FORCE (LBS)
FR-TO	FROM TO	FR-TO	FROM TO
A-B	-1350 / 0	-78.0	-78.0
B-C	-1316 / 0	-85.5	-85.5
C-D	-1316 / 0	-85.5	-85.5
D-E	-1316 / 0	-85.5	-85.5
E-F	-1316 / 0	-85.5	-85.5
F-G	-1316 / 0	-85.5	-85.5
G-H	-1316 / 0	-85.5	-85.5
H-I	-1316 / 0	-85.5	-85.5
I-J	-1316 / 0	-85.5	-85.5
J-K	-1316 / 0	-85.5	-85.5
K-L	-1316 / 0	-85.5	-85.5
L-M	-1316 / 0	-85.5	-85.5
M-N	-1316 / 0	-85.5	-85.5
N-O	-1316 / 0	-85.5	-85.5
O-P	-1316 / 0	-85.5	-85.5
P-Q	-1316 / 0	-85.5	-85.5
Q-R	-1316 / 0	-85.5	-85.5
R-S	-1316 / 0	-85.5	-85.5
S-T	-1316 / 0	-85.5	-85.5
T-U	-1316 / 0	-85.5	-85.5
U-V	-1316 / 0	-85.5	-85.5
V-W	-1316 / 0	-85.5	-85.5
W-X	-1316 / 0	-85.5	-85.5
X-Y	-1316 / 0	-85.5	-85.5
Y-Z	-1316 / 0	-85.5	-85.5

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per CBC 9.23.13.11, and no less than 2x4 for Part 4 design

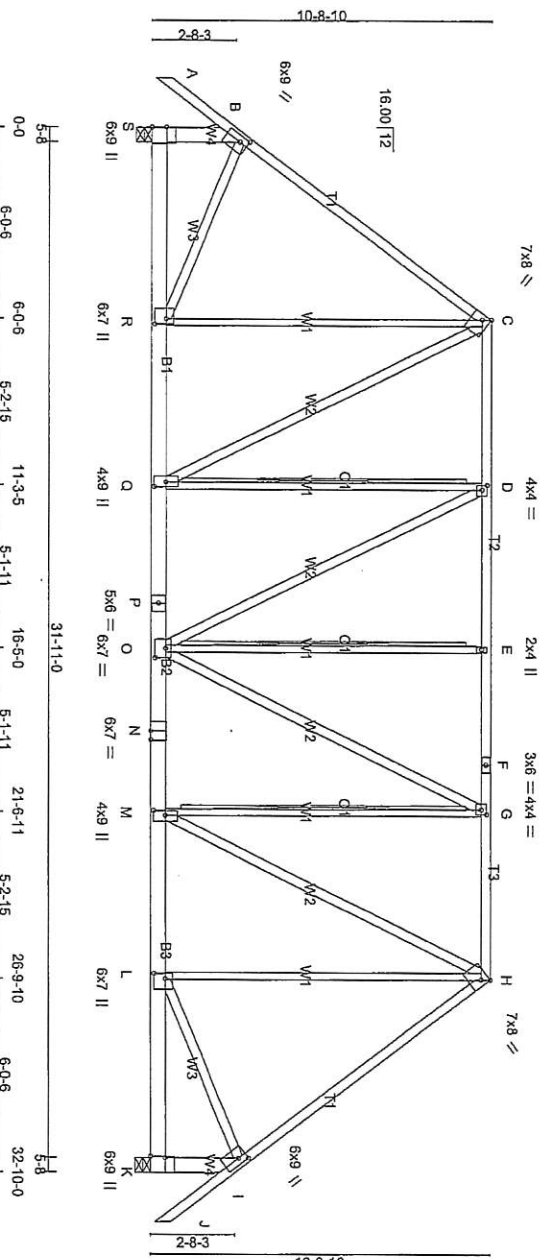


A-18023507

Version 8.200 S Jan 6 2018 MTEK Industries, Inc. Sat Feb 17 13:24:28 2018 Page 1

Version 8.200 S Jan 6 2018 MITEK Industries, Inc. Sat Feb 17 13:24:28 2018 Page 1

Scale = 1:69.4

[illegible]

Alpha Roof Truss, Maple

Version 8.200 S Jan 6 2018 MITek Industries, Inc. Sat Feb 17 13:24:29 2018 Page 2
ID:MF8litatTok7Dlbp1T4B71Xvs4PG-KRER271tOUTchlf-SxmF-XMnPHIHz83mFb02nzkEOW

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMWV-1	6.0	9.0	2.25	3.00
C	TMWV-1	7.0	8.0	Edge	2.75
D	TMWV-1	4.0	4.0	2.00	1.75
E	TMWV-1	2.0	4.0		
F	TS-1	3.0	6.0		
G	TMWV-1	4.0	4.0	2.00	1.75
H	TMWV-1	7.0	8.0	Edge	2.75
I	TMWV-1	6.0	9.0	2.25	3.00
K	BMV1+1	6.0	9.0	Edge	0.60
L	BMWV+1	6.0	7.0	4.25	2.00
M	BMWV+1	4.0	9.0	4.50	1.75
N	BS-1	6.0	7.0		
O	BMWVWV-1	6.0	7.0	4.00	3.50
P	BS-1	5.0	6.0		
Q	BMWV+1	4.0	9.0	4.50	1.75
R	BMWV+1	6.0	7.0	4.25	2.00
S	BMV1+1	6.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4
for Part 9 design as per OBC 9.23.13.11,
and no less than 2x4 for Part 4 design



A18023508(1)

5x6 //

2x4 //

5x6 //

16.00 T2

T1

T2

W1

W2

V1

V2

B1

B2

A

B

C

D

E

F

G

3x4 =

2x4 //

4x6 =

3x4 =

2x4 //

3x4 =

0-0

3-9-0

3-9-0

6-7-11

19-9-15

10-4-10

6-7-11

17-0-5

3-9-0

20-9-5

5-0-0

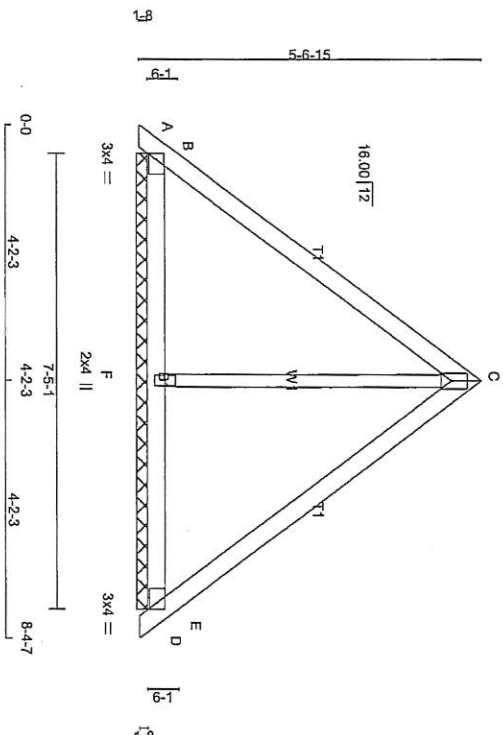
4-10-8

6-1

6-1

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292593	H23P	4	1	TRUSS DESC.	
Alpa Roof Truss, Maple					

ID: MF8l1at0k7Dl0p14B71Xys4PG-pwJEqHy_ofK9w_SGAIEG_KG2EYca0HkV2n1ZKEOE
Version 8.200 S Jan 6 2018 Mitek Industries, Inc. Sat Feb 17 13:24:47 2018 Page 1
7-10-15 8-4-7
Scale = 1/32"



TOTAL WEIGHT = 4 X 28 = 113 lb

LUMBER	N.L.G.A. RULES	SIZE	LUMBER	DESCR.
CHORDS	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY, SEASONED LUMBER.				

PLATES (Table is in inches)	W	LEN	Y	X
JT TYPE	3.0	4.0	1.50	2.75
TM81-I	MT20	3.0	5.0	2.00
C	TTW+P	MT20	3.0	4.0
D	TM81-I	MT20	3.0	4.0
F	BMW1+W	MT20	2.0	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REARD
JT	GROSS REACTION	DOWN	HORIZ	UP/LIFT
B	277	0	277	0
D	277	0	277	0
F	216	0	216	0

UNFACTORED REACTIONS	1ST LCASE	MAX. MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
JT	COMBINED	SNOW	LIVE	PERM. LIVE	
B	195	135/0	0/0	0/0	59/0
D	195	135/0	0/0	0/0	59/0
F	159	70/0	0/0	0/0	89/0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	MAX. FACTORED	W E B S	MAX. FACTORED
MEMB.	FORCE	VERT. LOAD LC1	MAX. CS1 (LC)	MEMB.	FORCE
FR-TO	(LBS)	FROM TO	LENGTH FR-TO	F-C	130/0
A-B	0/10	-78.0	0.01 (1)	10.00	0.06 (1)
B-C	-98/0	-78.0	0.18 (1)	6.25	
C-D	-98/0	-78.0	0.18 (1)	6.25	
D-E	0/10	-78.0	0.01 (1)	10.00	
B-F	0/58	-18.5	-18.5 0.08 (4)	10.00	
F-D	0/58	-18.5	-18.5 0.08 (4)	10.00	

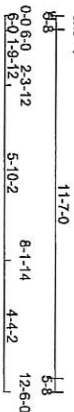
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015
THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-08, CSA 086-14
- TPI 2011, TPI 2014
(55% OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD
CS1: TO=0, 18/1.00 (B-C:1), BC=0.08/1.00 (B-F:4), WB=0.05/1.00 (C-F:1), SS1=0.07/1.00 (B-C:1)
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10
COMPANION LIVE LOAD FACTOR = 1.00
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.
NAIL VALUES
PLATE GRIP (PSI) SHEAR SECTION (PSI) (PL) (PL) MAX MIN MAX MIN MT20 618 354 1687 788 1987 1656
PLATE ROTATION TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.
JSI GRIP= 0.33 (D) (INPUT = 0.90)
JSI METAL= 0.12 (D) (INPUT = 1.00)

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



A-18023511

Version 8.200 S Jan 6 2018 MITRE Industries, Inc. Sat Feb 17 13:25:15 2018 Page 1
ID-MF:BillataToK/Dlbp14B71Xys4PG_H0qlrYtCkYglPPYx6m77DQnqz7Am5sdqLPKs5zEzEnC
8-1-14 3-10-10 5-9 12-28 12-6-0 5-10-2 0-5-8 2-3-12 0-0-5-8
5-8-1-10-4-1 3x5 II Scale = 1:73.3



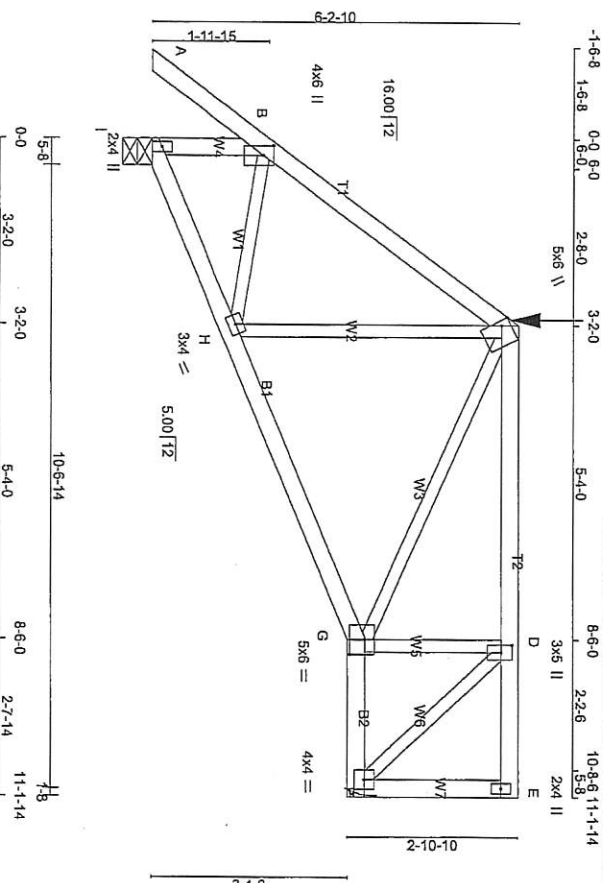
DESIGN CRITERIA	
SPECIFIED LOADS:	
TOP CH. LL	21.0 PSF
DL	6.0 PSF
BOT CH. LL	0.0 PSF
DL	7.4 PSF
TOTAL LOAD	34.4 PSF
SPACING = 24.0 IN. O.C.	

- FRN 100-00, CSA 2012, Q100 2010
 - CSA 086-09, CSA 086-14
 - TPC 2011, TPC 2014
 (66 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F.
 RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED
 ROOF LIVE LOAD
 ALLOWABLE DEF'L (DL) = U360 (0.42")
 CALCULATED VERT. DEF'L (DL) = U 939 (0.02")
 ALLOWABLE DEF'L (TL) = U360 (0.42")

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

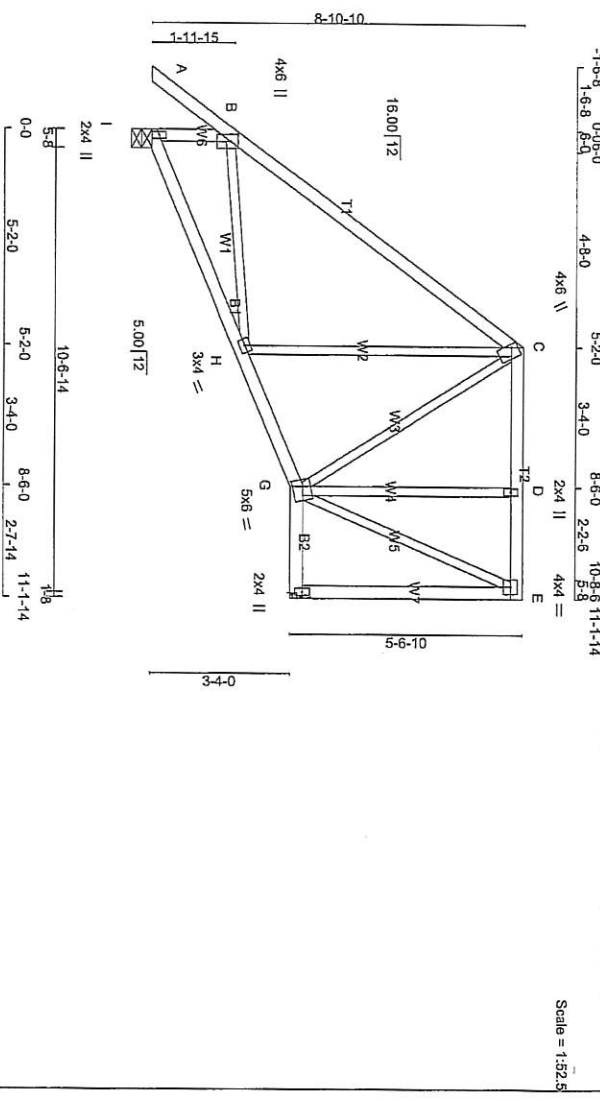
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292593	H26	1	1	TRUSS DESC.	
Alpha Roof Truss, Maple					

Version 6.200 5 Jan 6 2018 Mitek Industries, Inc. S&P Feb 17 13:26:44 2018 Page 1
ID:M\Filina Tok7Dbpt14B71Xys4Pg-4PUTNHNNFHuJXhmsOD7c_3V/29mwolcRzKEMP
10-6-11-1-14
Scale = 1:37.3



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
292593	H27	1	1			
Alpha Roof Truss, Maple						

Version 8.200 5 Jan 6 2018 Mitek Industries, Inc. Sat Feb 17 13:29:56 2018 Page 1
ID:MF8uiraTok7DIDpT4B71Xys4P-G-GLU2gxp1G44ZxNZvdJR7UPzPC52_J7bByd1q1JzKEJF



LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA			
N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.	BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	RECORD	SPECIFIED LOADS:	TOTAL WEIGHT = 63 lb (M)
A - C	2x4	DRY	No.2	SPF	VERT	GROSS REACTION	DOWN	BRG	BRG	TOP CH. LL = 21.0 PSF	
C - E	2x4	DRY	No.2	SPF	VERT	GROSS REACTION	UP	BRG	BRG	TOP CH. LL = 6.0 PSF	
F - E	2x4	DRY	No.2	SPF	VERT	GROSS REACTION	DOWN	BRG	BRG	BOT CH. LL = 0.0 PSF	
I - B	2x4	DRY	No.2	SPF	VERT	GROSS REACTION	UP	BRG	BRG	BOT CH. LL = 7.4 PSF	
I - G	2x4	DRY	No.2	SPF	VERT	GROSS REACTION	DOWN	BRG	BRG	TOTAL LOAD = 34.4 PSF	
G - F	2x4	DRY	No.2	SPF	VERT	GROSS REACTION	UP	BRG	BRG		
ALL WEBS	2x3	DRY	No.2	SPF	VERT	GROSS REACTION	DOWN	BRG	BRG		
EXCEPT											
DRY, SEASONED LUMBER.											

PLATES (table is in inches)				UNFACTORED REACTIONS				LOADING IN PLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM			
JT TYPE	PLATES	W	LEN Y X	1ST LOUSE	SNOW	LIVE	PERMANENT	WIND	DEAD	SOIL	
B	2x4	4.0	6.0	2.00	2.00	0.0	0.0	0.0	150.0	0.0	
C	2x4	4.0	6.0	1.75	1.00	0.0	0.0	0.0	165.0	0.0	
E	2x4	4.0	6.0	1.75	1.00	0.0	0.0	0.0	165.0	0.0	
F	2x4	4.0	6.0	1.75	1.00	0.0	0.0	0.0	165.0	0.0	
G	2x4	4.0	6.0	1.75	1.00	0.0	0.0	0.0	165.0	0.0	
H	2x4	4.0	6.0	1.75	1.00	0.0	0.0	0.0	165.0	0.0	
I	2x4	4.0	6.0	1.75	1.00	0.0	0.0	0.0	165.0	0.0	

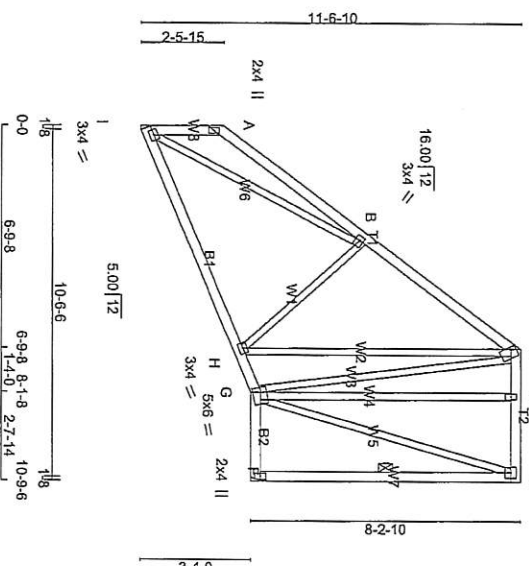
CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CS1 (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0.43	-76.0	0.11 (1)	H-C	0.81	0.03 (4)	ALLOWABLE DEF. (L) = L/260 (0.37")
B-C	-392.0	-76.0	0.38 (1)	C-G	-27.0	0.02 (4)	CALCULATED VERT. DEF. (L) = U/999 (0.01")
C-D	-220.0	-76.0	0.11 (1)	G-D	-286.0	0.13 (1)	ALLOWABLE VERT. DEF. (L) = L/260 (0.37")
D-E	-220.0	-76.0	0.11 (1)	G-E	0.500	0.11 (1)	ALLOWABLE VERT. DEF. (L) = U/999 (0.03")
E-F	-514.0	0.0	0.028 (1)	B-H	0.235	0.05 (1)	
F-G	-606.0	0.0	0.007 (1)				CSI: TC=0.387,0.00 (B-C:1), BC=0.137,0.00 (G-H:4) WB=0.137,0.00 (D-G:1), SS=0.121,0.00 (C-D:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10
G-H	0.4	-18.5	0.11 (4)				COMPANION LIVE LOAD FACTOR = 1.00
H-G	0.248	-18.5	0.13 (4)				
G-F	0.0	-18.5	0.04 (4)				TRUSS PLATE MANUFACTURER IS NOT

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



A-18023515

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292593	H28	1	1	TRUSS DESC.	
ID:MF8l1iaTck7Dlbp14871Xys4PG-zG5qmMq9dLlmU8UJUnFbzZf7Sw2RWfEjLNIckEJF Version 8.200 S Jan 6 2018 Mitek Industries, Inc. Sat Feb 17 13:30:06 2018 Page 1 6-9-8 8-1-8 10-3-14 10-9-6 1-8 3-5-0 3-6-8 3-3-0 1-4-0 2-2-6 5-8 0-0-1-8 4x6 // 2x4 // 4x4 =					
Apea Root Truss, Maple Scale = 1/67.0					



LUMBER	N.L.G.A. RULES	LUMBER	DESCR	DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY						DESIGN CRITERIA			
CHORDS	SIZE			BUILDING MEMBER						SPECIFIED LOADS:			
				BEAMS		MAXIMUM FACTORED		INPUT		RECORD			
A - C	2x4	No.2	SPF	FACTORED	GROSS REACTION	GROSS REACTION	DOWN	UP/LIFT	BRG.	IN-SX	TOP CH. DL =	21.0	PSF
C - E	2x4	No.2	SPF	VERT	HORIZ	HORIZ	0	0	HANGER BY OTHERS	IN-SX	TL =	6.0	PSF
F - E	2x4	No.2	SPF	F	0	0	0	0	HANGER BY OTHERS	IN-SX	BOT CH. DL =	0.0	PSF
I - A	2x4	No.2	SPF						MIN. SEAT SIZE: 1-8		TL =	7.4	PSF
I - G	2x4	No.2	SPF						HANGER BY OTHERS		TOTAL LOAD =	34.4	PSF
G - F	2x4	No.2	SPF	I	520	0	520	0	0	MIN. SEAT SIZE: 1-8			
ALL WEBS EXCEPT	2x3	No.2	SPF	UNFACTORED REACTIONS									
				1ST LAYSE		MAX. MIN. COMPONENT REACTIONS						LOADING IN FLAT SECTION BASED ON A	
										SPACING =		24.0	IN. C/C

Alpha Roof Truss, Maple

QUANTITY	PLY
1	1

DRWG NO.

0-0 1-8 4-6-8 8-9-8 10-3-14 10-9-6
1-8 4-5-0 1-6-6 5-8

Structural drawing of a roof truss system. The drawing includes the following dimensions and labels:

- Top Dimensions:**
 - 14-2-10 (Total length)
 - 2-5-15 (Segment length)
- Left Side Dimensions:**
 - 10-6-5 (Total height)
 - 10-9-6 (Segment height)
 - 8-1-8 (Segment height)
 - 3-7-0 (Segment height)
 - 4-6-8 (Segment height)
 - 4-6-8 (Segment height)
 - 0-0 (Segment height)
- Internal Dimensions:**
 - 5.00 | 12 (Internal horizontal dimension)
 - 16.00 | 12 (Internal horizontal dimension)
 - 3x4 = (Internal vertical dimension)
 - 3x4 = (Internal vertical dimension)
 - 5x6 = (Internal vertical dimension)
 - 3x4 = (Internal vertical dimension)
- Labels:**
 - H** (at the top left)
 - 2x4 ||** (along the left edge)
 - 4x6 ||** (along the top edge)
 - A** (at the top left corner)
 - B1** (at the top right corner)
 - F** (at the bottom left corner)
 - B2** (at the bottom right corner)
 - W1** (at the bottom left corner)
 - W2** (at the bottom right corner)
 - W3** (at the bottom right corner)
 - W4** (at the bottom right corner)
 - W5** (at the bottom right corner)
 - W6** (at the bottom right corner)
 - W7** (at the bottom right corner)
 - G** (at the bottom right corner)
- Bottom Dimensions:**
 - 10-10-10 (Total length)
 - 3-4-0 (Segment length)

DESIGN CRITERIA	
SPECIFIED LOADS:	
TOP CH. LL =	21.0 PSF
DL =	6.0 PSF
BOT CH. LL =	0.0 PSF
DL =	7.4 PSF
TOTAL LOAD =	34.4 PSF
SPACING = 24.0 IN./C/C	

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRID(DRY)	SHEAR (psi)	SECTION (PL)
MAX	MIN	MAX	MIN
MT20	618	354	1687
	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

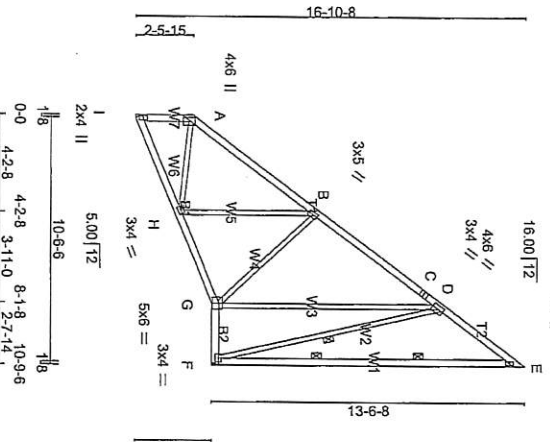
JSI GRIP= 0.46 (E) (INPUT = 0.90)
JSI METAL= 0.17 (H) (INPUT = 1.00)

A-18023517

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	PRWG NO.
292593	H30	2	1			
Alpha Roof Truss, Maple						

ID:MFB8lhaTck/Dbp14871Xys4PG-on_XcdSWFPEOD0Puk3PBtrGVfPK4Wt2YGo3guZKElu
 0-0-1-8 4-2-8 3-11-0 8-1-8 10-3-14 10-9-6
 1-8 4-1-0 2-2-5-8 2-4 II

Scale: 1/8"=1'



TOTAL WEIGHT = 2 X 79 = 158 lb [M/F]

LUMBER	N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.	DESIGN CRITERIA
I - A	2x4	DRY	No.2	SPF	SPF	SPECIFIED LOADS: TOP CH. LL = 21.0 PSF BOT CH. LL = 0.0 PSF DL = 7.4 PSF TOTAL LOAD = 34.4 PSF
A - E	2x4	DRY	No.2	SPF	SPF	SPACING = 24.0 IN. C/C
C - E	2x4	DRY	No.2	SPF	SPF	THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015
F - E	2x4	DRY	No.2	SPF	SPF	THIS DESIGN COMPLIES WITH: -PART 9 OF OBC 2012, OBC 2018 -CSA 086-08, CSA 086-14 -TPIC 2011, TPIC 2014
G - F	2x4	DRY	No.2	SPF	SPF	(65 % OF 23.0 P.S.F., G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD
ALL WEBS	2x3	DRY	No.2	SPF	SPF	
DRY: SEASONED LUMBER.						

UNFACTORED REACTIONS	1ST LOOSE	MAX MIN. COMPONENT REACTIONS	FACTORED REACTIONS	MAXIMUM FACTORED	INPUT	REGRD
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
I	3/1	226 / 0	0 / 0	0 / 0	0 / 0	144 / 0
F	3/1	226 / 0	0 / 0	0 / 0	0 / 0	144 / 0

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2 LATERAL BRACE(S) AT 1/3 LENGTH OF E-F.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-F.

CHORDS	MEMB.	FORCE	VERT. LOAD	FACTORED	MAX. UNBRACED	MEMB.	MAX. FACTORED
FR-TO							
I-A	-489 / 0	0.0	0.07 (1)	7.81	G-D	0 / 310	0.07 (1)
A-B	-393 / 0	-78.0	0.16 (1)	6.25	D-F	-440 / 0	0.33 (1)
B-C	-205 / 0	-78.0	0.15 (1)	6.25	A-H	0 / 263	0.06 (1)
C-D	-205 / 0	-78.0	0.15 (1)	6.25	H-B	-36 / 54	0.02 (1)
D-E	-23 / 0	-78.0	0.08 (1)	6.25	B-G	-206 / 0	0.13 (1)
F-E	-70 / 0	0.0	0.03 (1)	6.25			
I-H	0 / 4	-18.5	-18.5 0.09 (4)	10.00			
H-G	0 / 269	-18.5	-18.5 0.11 (4)	10.00			
G-F	0 / 111	-18.5	-18.5 0.05 (4)	10.00			

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

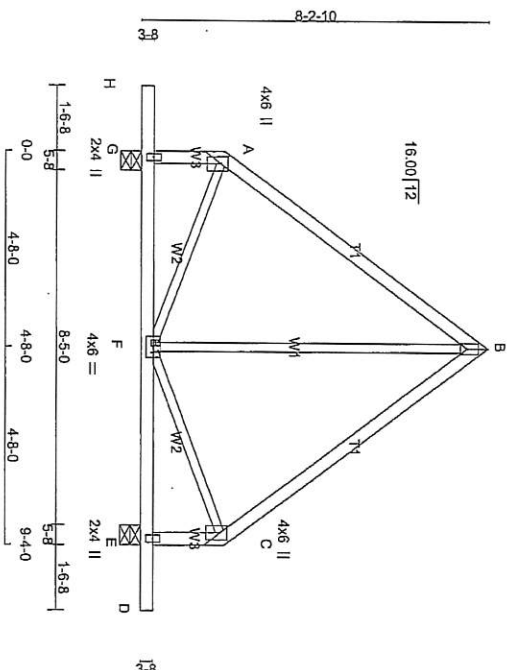


A-18023518

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG. NO.
292593	H31	3	1	TRUSS DESC.	
Alpha Roof Truss, Maple					

Version 6.200 S Jan 6 2018 Mittek Industries, Inc. Sat Feb 17 13:30:36 2018 Page 1
 ID:MF6llnat0k7Dlbp14871Xy64PG-52V44GBvzC7PM65RF11qdwDgdDl0374687XQ_ZKEH
 -1-6-8 0-0 5-8 4-2-6 4-8-0 4-2-6 8-10-8 9-4-0 10-10-8
 1-6-8 5-8 4-2-6 3x5 II 8-10-8 9-4-0 10-10-8

Scale = 1:52.2



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF	
B - C	2x4	DRY	No.2	SPF	
G - A	2x4	DRY	No.2	SPF	
E - C	2x4	DRY	No.2	SPF	
H - D	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
DRY: SEASONED LUMBER.					

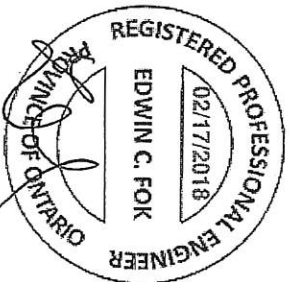
PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
A	TWV+P	MT20	4.0	6.0	2.00	2.00
B	TWV+P	MT20	3.0	5.0	2.00	Edge
C	TWV+P	MT20	4.0	6.0	2.00	2.00
E	BMV1+P	MT20	2.0	4.0		
F	BMV+P	MT20	4.0	6.0		
G	BMV1+P	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
 TOUCHES EDGE OF CHORD.

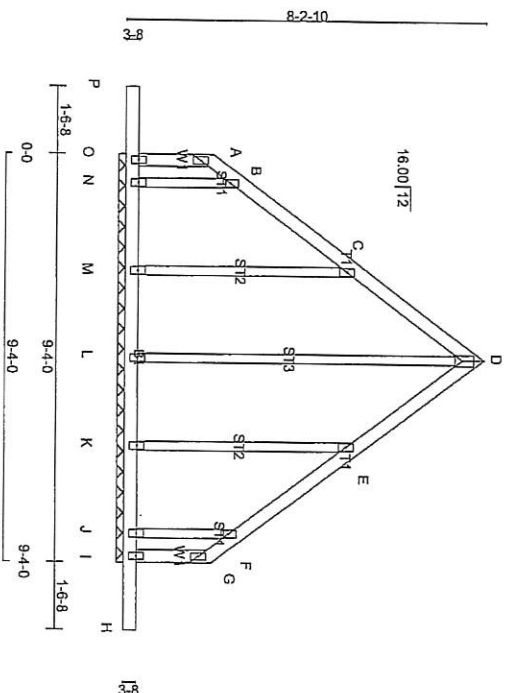
DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
FACTORED	MAXIMUM FACTORED	INPUT	RECORD						
GROSS REACTION	GROSS REACTION	BRG	BRG						
JT VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX				
E	599	0	599	0	5-8	1-8			
UNFACTORED REACTIONS									
1ST LCLASE MAX MIN. COMPONENT REACTIONS									
JT	COMBINED	SNOW	LIVE	PERMALIVE	WIND	DEAD	SOIL		
G	427	261/0	0/0	0/0	0/0	166/0	0/0		
E	427	261/0	0/0	0/0	0/0	166/0	0/0		
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, E									
BRACING									
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.									
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.									
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.									
LOADING									
TOTAL LOAD CASES: (4)									
CHORDS				WEBS					
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT LOAD (PLF)	UNBRACED LENGTH (FT)	MAX. MEMB. FORCE (LBS)	MAX. FACTORED CS1 (L/C)	MAX. CS1 (L/C)			
FR-TO	-234/0	-78.0	0.22 (1)	6.25	F-B	0.79	0.03 (4)		
B-C	-234/0	-78.0	0.22 (1)	6.25	A-F	0.148	0.03 (1)		
G-A	-417/0	0.0	0.05 (1)	7.81	F-C	0.148	0.03 (1)		
E-C	-417/0	0.0	0.05 (1)	7.81					
H-G	0/0	-96.5	0.16 (1)	10.00					
G-F	0/0	-18.5	0.11 (4)	10.00					
F-E	0/0	-18.5	0.11 (4)	10.00					
E-D	0/0	-96.5	0.16 (1)	10.00					

DESIGN CRITERIA			
SPECIFIED LOADS:	TOP CH.	LL = 21.0	PSF
	DL = 6.0	PSF	
	BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF	
TOTAL LOAD	= 34.4	PSF	
SPACING	= 24.0	IN. C/C	
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015			
THIS DESIGN COMPLIES WITH:			
- PART 9 OF OBC 2012, OBC 2018			
- CSA 086-09, CSA 086-14			
- TPIC 2011, TPIC 2014			
(65 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD			
ALLOWABLE DEF.(LL) = L/360 (0.31")			
CALCULATED VERT DEF.(LL) = L/369 (0.00")			
ALLOWABLE DEF.(TL) = L/360 (0.31")			
CALCULATED VERT DEF.(TL) = L/369 (0.01")			
CANTILEVER DEFLECTION:			
ALLOWABLE DEF.(LL) = L/120 (0.19")			
CALCULATED VERT DEF.(LL) = L/999 (0.01")			
ALLOWABLE DEF.(TL) = L/120 (0.19")			
CALCULATED VERT DEF.(TL) = L/999 (0.01")			
CS1-TG=0.224 (0.8-C-1), BC=0.161 (0.12-E-1), WB=0.037 (1.00 C-1-1), SS=0.124 (1.00 G-H-1)			
DOI LUMBER=1.00 NAL=1.00 LS BEND=1.10			
COMP=1.10 SHEAR=1.10 TENS=1.10			
COMPANION LIVE LOAD FACTOR = 1.00			
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.			
NAL VALUES			
PLATE GRIP(ORY)	SHEAR	SECTION	
(P8)	(PL)	(PL)	
MAX MIN	MAX MIN	MAX MIN	
MT20	618	354	1667
	788	1987	1656
PLATE PLACEMENT TOL. = 0.260 inches			
PLATE ROTATION TOL. = 5.0 Deg.			
JSI GRIP= 0.24 (A) (INPUT = 0.90)			
JSI METAL= 0.12 (C) (INPUT = 1.00)			

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



A-18023519



TOTAL WEIGHT = 54.1b

LUMBER	N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
O - A	2x4	DRY	No.2	SPF	
A - D	2x4	DRY	No.2	SPF	
D - G	2x4	DRY	No.2	SPF	
I - G	2x4	DRY	No.2	SPF	
P - H	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
ALL GABLE WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					
GABLE STUDS SPACED AT	2-0-0 OC.				

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

PROVIDE ANCHORAGE AT BEARING JOINT N FOR 150 LBS. FACTORED UPLIFT

PROVIDE ANCHORAGE AT BEARING JOINT J FOR 150 LBS. FACTORED UPLIFT

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINTS).

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

DESIGN CRITERIA
SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
TOP CH. LL = 6.0 PSF
BOT CH. LL = 0.0 PSF
BOT CH. LL = 7.4 PSF
TOTAL LOAD = 34.4 PSF
SPACING = 24.0 IN. C/C
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015
THIS DESIGN COMPLIES WITH:
-PART 9 OF OBC 2012, OBC 2018
-CSA 086-09, CSA 086-14
-TPIC 2011, TPIC 2014
DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.
(65 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD
CS: TC=0.04/1.00 (B-C-1), BC=0.16/1.00 (H-I-1), WB=0.22/1.00 (D-E-1), SS=0.14/1.00 (I-L-1)
DOL LUMBER=1.00 NAIL=1.00 IS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10
COMPANION LIVE LOAD FACTOR = 1.00

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE				
A TMV+P	M120	2.0	4.0	
B, C, E, F	M120	2.0	4.0	
D TMV+P	M120	3.0	5.0	2.00 Edge
G TMV+P	M120	2.0	4.0	
I BMV+P	M120	2.0	4.0	
J, K, L, M, N	M120	2.0	4.0	
O BMV+P	M120	2.0	4.0	

CHORDS	MEMB. FACTORED FORCE (LBS)	VERT. LOAD LC1 (PLF)	MAX. FACTORED CSI (L/C)	MAX. UNBRAC LENGTH (FT)	MEMB. FACTORED FORCE (LBS)	MAX. FACTORED CSI (L/C)
FR-TO						
O-A	-20.0	0.0	0.02 (1)	7.81	L-D	0.22 (1)
A-B	0.5	-78.0	0.02 (1)	10.00	M-C	-178.0
B-C	0.10	-78.0	0.04 (1)	10.00	N-B	-90.0
C-D	0.9	-78.0	0.04 (1)	10.00	K-E	-165.0
D-E	0.9	-78.0	0.04 (1)	10.00	J-F	-90.0
E-F	0.10	-78.0	0.04 (1)	10.00		
F-G	0.5	-78.0	0.02 (1)	10.00		
G	-20.0	0.0	0.02 (1)	7.81		
P-O	0.0	-95.5	0.16 (1)	10.00		
O-N	-3.2	-18.5	0.14 (1)	10.00		
N-M	-4.0	-18.5	0.02 (4)	10.00		
M-L	-6.0	-18.5	0.02 (4)	10.00		
L-K	-6.0	-18.5	0.02 (4)	10.00		
K-J	-4.0	-18.5	0.02 (4)	10.00		
J-I	-3.2	-18.5	0.14 (1)	10.00		
I-H	0.0	-95.5	0.16 (1)	10.00		

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAVL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PL) (PL)

MAX MIN MAX MIN

M120 618 354 1667 708 1907 1656

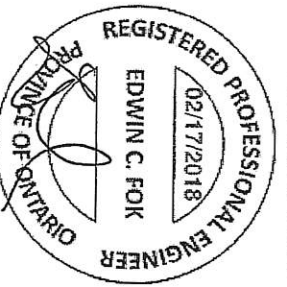
PLATE PLACEMENT TOL. = 0.260 inches

PLATE ROTATION TOL. = 5.0 Deg.

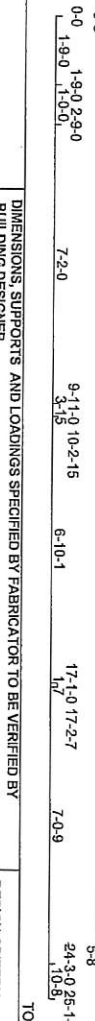
JSI GRIP= 0.17 (D) (NPUT = 0.90)

JSI INETAL= 0.06 (E) (NPUT = 1.00)

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



A-18023520

[illegible]

FR-TO	FROM	TO	LENGTH	FR-TO	(in)	(in)		
A-O	-1682/0	-780	-780	0.11 (1)	5.04	M-B	0.149	0.03 (1)
O-B	-1591/0	-780	-780	0.12 (1)	5.15	M-C	0.734	0.17 (1)
B-C	-1441/0	-780	-780	0.12 (1)	5.34	L-C	-743/0	0.17 (1)
C-D	-151/0	-780	-780	0.14 (1)	5.75	H-G	-1185/0	0.57 (1)
D-E	-997/0	-780	-780	0.13 (1)	6.12	K-E	0.290	0.07 (1)
E-F	-751/0	-855	-855	0.54 (1)	2.00	K-F	0.111	0.02 (1)
F-G	-687/0	-855	-855	0.54 (1)	2.00	L-F	-827/0	0.56 (1)
						I-F	0.1183	0.19 (1)
A-N	0/1143	-18.5	-18.5	0.22 (1)	10.00	L-D	0.718	0.01 (4)
N-M	0/1143	-16.5	-18.5	0.22 (1)	10.00	D-K	-271/0	0.31 (1)
M-L	0/1179	-16.5	-18.5	0.19 (1)	10.00	N-O	0/82	0.00 (1)
L-K	0/882	-16.5	-18.5	0.32 (4)	10.00			
K-J	0/887	-16.5	-18.5	0.31 (4)	10.00			
J-I	0/887	-16.5	-18.5	0.31 (4)	10.00			
I-H	0/0	-18.5	-18.5	0.19 (4)	10.00			

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

CS1: TC=0.54/1.00 (E-F-1), BC=0.32/1.00 (K-L-4), WB=0.57/1.00 (G-H-1), SH=0.29/1.00 (E-F-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PL) (PL)



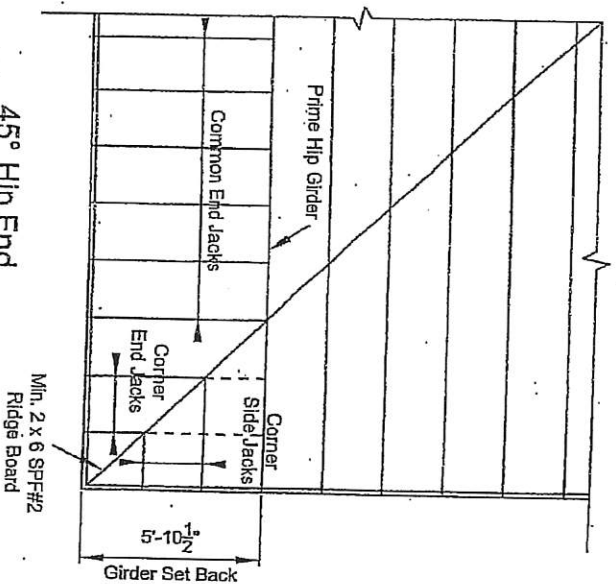
STRACON ENGINEERING INC.

LUMBER SPECIFICATION

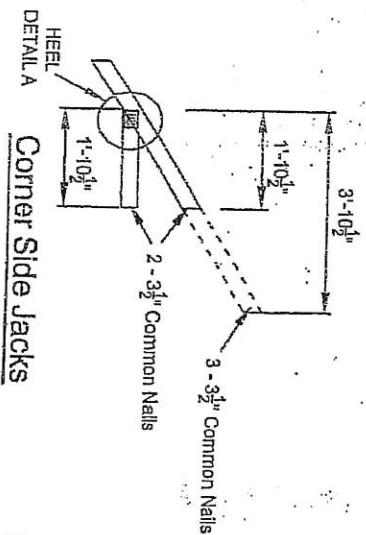
TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

DESIGN LOAD

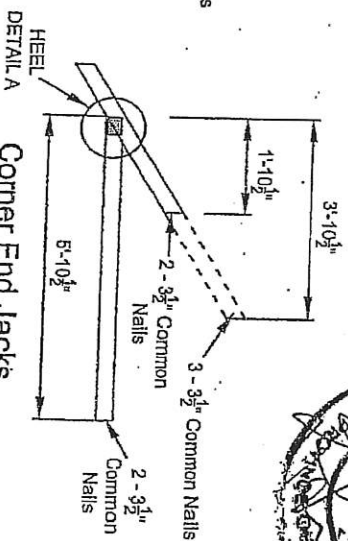
TOP CHORD SNOW LOAD	: 40.5 P.S.F.
TOP CHORD DEAD LOAD	: 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	: 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	: 7.0 P.S.F.
TOTAL LOAD	: 50.5 P.S.F



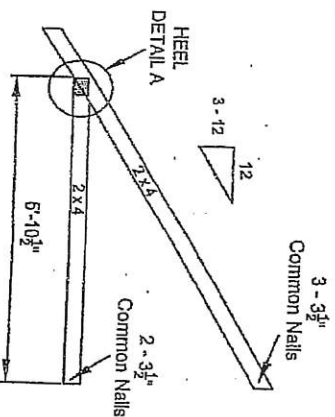
45° Hip End



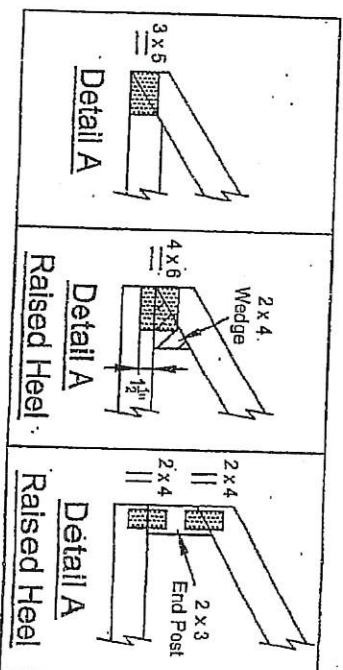
Corner Side Jacks



Corner End Jacks

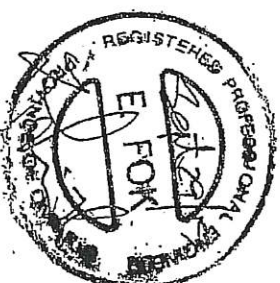


Common End Jacks

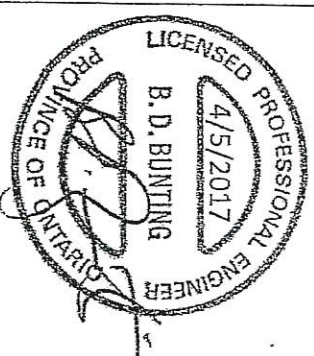


NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

CS-51008



A technical line drawing of a dovetail joint. It shows a rectangular block (the tenon) with a dovetail-shaped notch cut into its top surface. This block is being inserted into a larger rectangular block (the mortise) which has a corresponding dovetail-shaped hole. The drawing illustrates the interlocking nature of the joint.

 r, d_o is the distance from the seat of the hanger to the highest joist nail.

HUS/LJS – Double Shear Joist Hangers



All hangers have double shear nailing. This patented innovation

distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

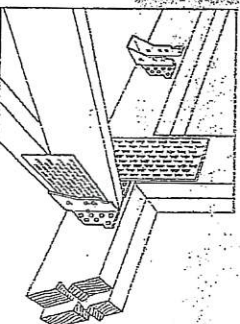
- Factored resistances are in accordance with CSA O86 -14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

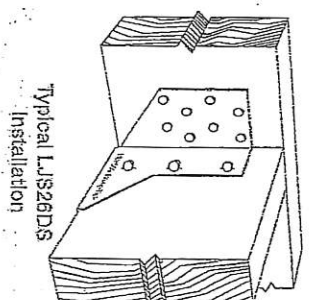
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

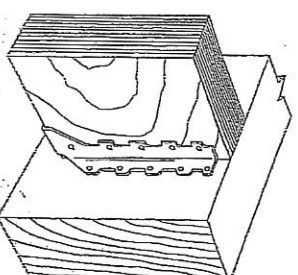
- See current catalogue for options



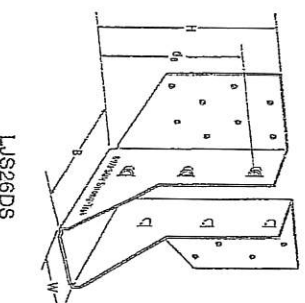
Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)



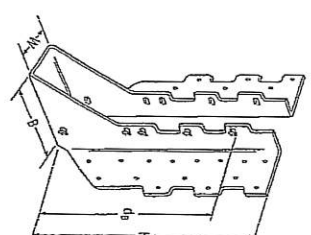
Typical LJS26DS Installation



Typical HUS Installation



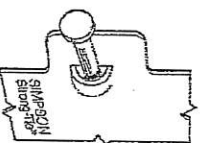
LJS26DS



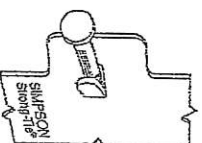
HUS210
(HUS26, HUS28, similar)

Model No.	Ga.	Dimensions (in.)				Fasteners				Factored Resistance (lb.)			
		W	H	B	d _h ¹	Face	Joist			D, E ¹ -L	S-P-E		
LJS26DS	18	1 9/16	5	3 1/2	4 5/8	(16) 16d	(6) 16d	Uplift (K _h =1.15)	Normal (K _h =1.00)	Uplift (K _h =1.15)	Normal (K _h =1.00)	lb.	lb.
LJS26	16	1 9/16	5 3/4	3	3 5/8	(14) 16d	(6) 16d	2055	4265	1460	4115		
HUS28	16	1 9/16	7 3/4	3	6 3/4	(22) 16d	(6) 16d	2705	4940	2065	3875		
HUS210	16	1 9/16	9 3/4	3	7 3/4	(30) 16d	(10) 16d	3605	6365	2675	4945		
HUS210	16	1 9/16	9 3/4	3	7 3/4	(30) 16d	(10) 16d	4505	6795	4010	4740		
HUS1, 81/10	16	1 1/2	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200		

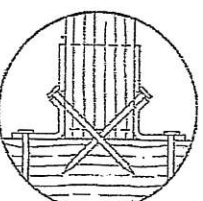
1. d_h is the distance from the seat of the hanger to the highest joist nail.



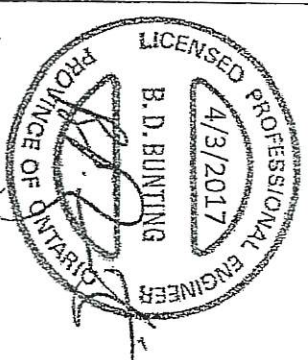
Dome Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 6,603,680



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT
STATES
DESIGN

This technical bulletin is effective until June 30, 2019, and related information available as of March 1, 2017. This information is updated periodically and is provided for related updates. July 30, 2016.
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(800) 999-5099
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