

Builder / Location: **GOLD PINE**
Project: **PINE**
Date: 1/22/2011

ARK HOMES
/ALLEY
Designer: AMANDA

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ALPA ROOF TRUSSES INC. AND WILL BE RETRACTED BY ALPA ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

Model / Elevation: SUB 02 OF 1 (LOGS/HAGUARY)
5006 REV1 / A

SILVERWOOD

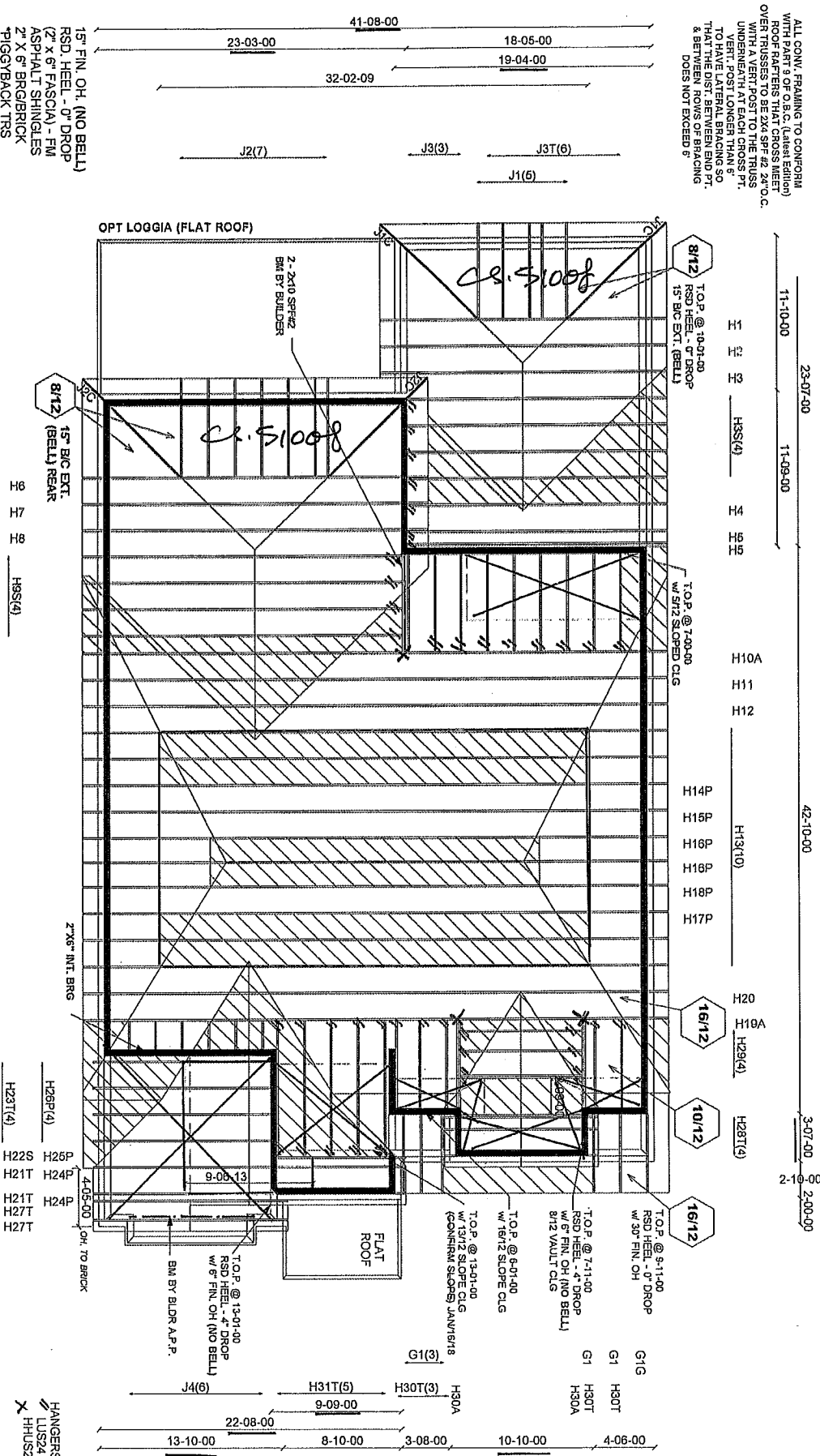
$$\text{STD} \propto \sigma_T \text{LOGGIA(FURT)}$$


CONVENTIONAL
FRAMING BY
OTHERS

PURLINS BY OTHERS
LOFT WALLS SUPPORTED BY OTHERS
CONFIRM TRUSS DIRECTION LAYOUT
P.L. 95914

OFFICE USE ONLY 63-00-00

HANGERS
// LUS24
X HHUS26-2



ALL CONJ. FRAMING TO CONFORM WITH PART 9 OF O.B.C. (Latest Edition) ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2X6 SPT #2 24"O.C. WITH A VERT.POST TO THE TRUSSE UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DIST. BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

ALPHA-BET

Job Track: 45147
Layout ID: 292662
Plan Log: 95914

Job Track **45147**

Layout ID: 292662

Plan Log: 95914

Builder / Location:

GOLD PARK HOMES / VAUGHAN

Project: **PINE VALLEY**

Date: 1/22/2018	Designer: AMANDA
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Model / Elevation: Silverwood

5006 REV1

1A OPT TRAY

OR OPT LOGGIA (FLAT)

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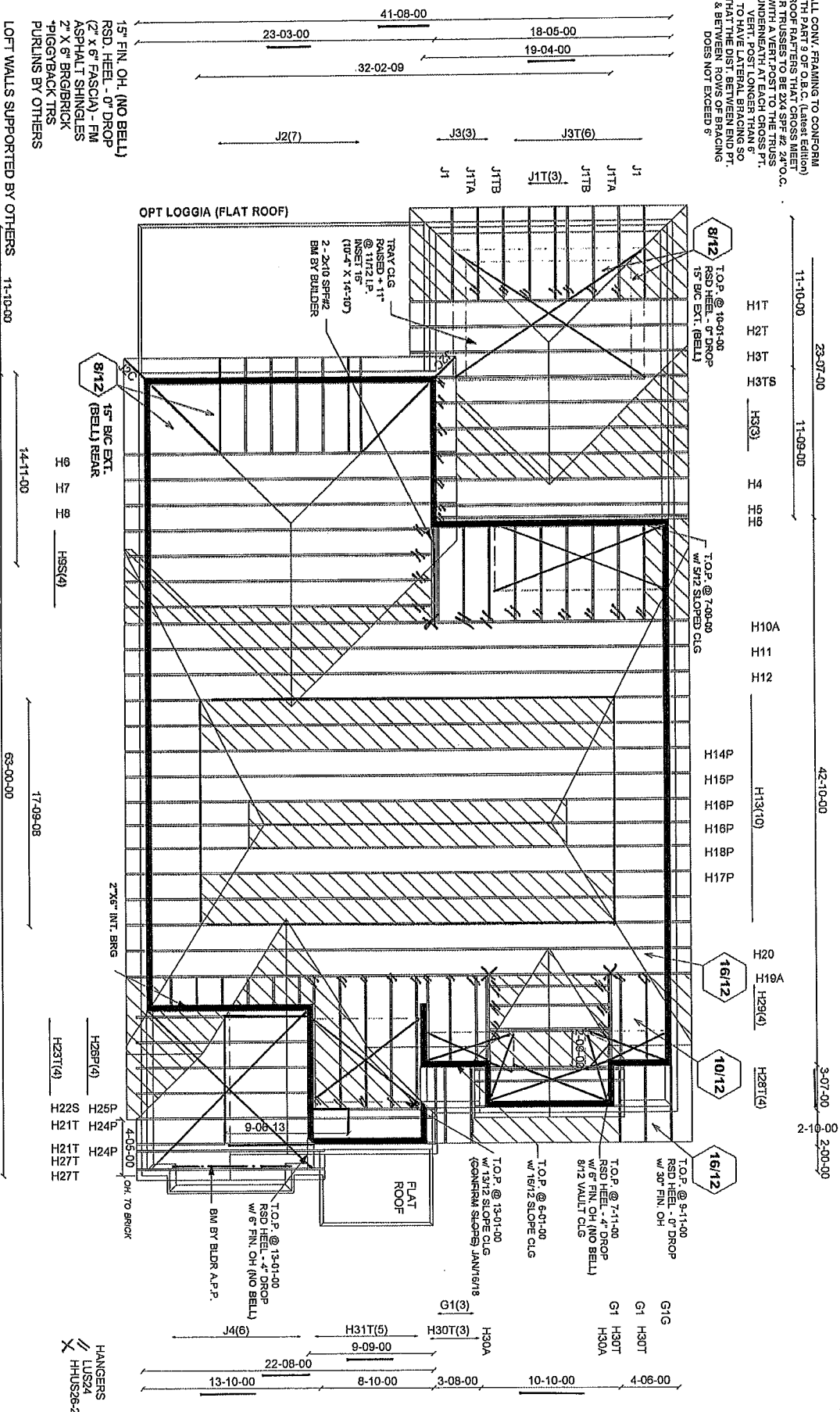
CONVENTIONAL
FRAMING BY
OTHERS

PURLINS BY OTHERS
LOFT WALLS SUPPORTED BY OTHERS
CONCREM TRUSS DIRECTIONAL LAYOUT
P.L. 95914

LOFT WALLS SUPPORTED BY OTHERS

CONFIRM KROSS DIRECTIONS/FAVOUR
P.L. 95914

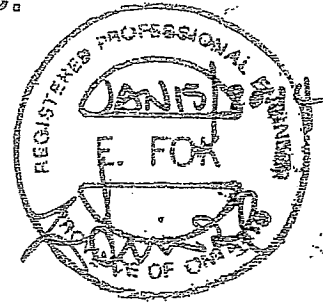
A-18023946
A-18023948 - A-18023984



ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. (Latest Edition)
ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2X4 SP# 2, 24"O.C. WITH A VERT.POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT.
VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DIST. BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

ALPA-BA-BE

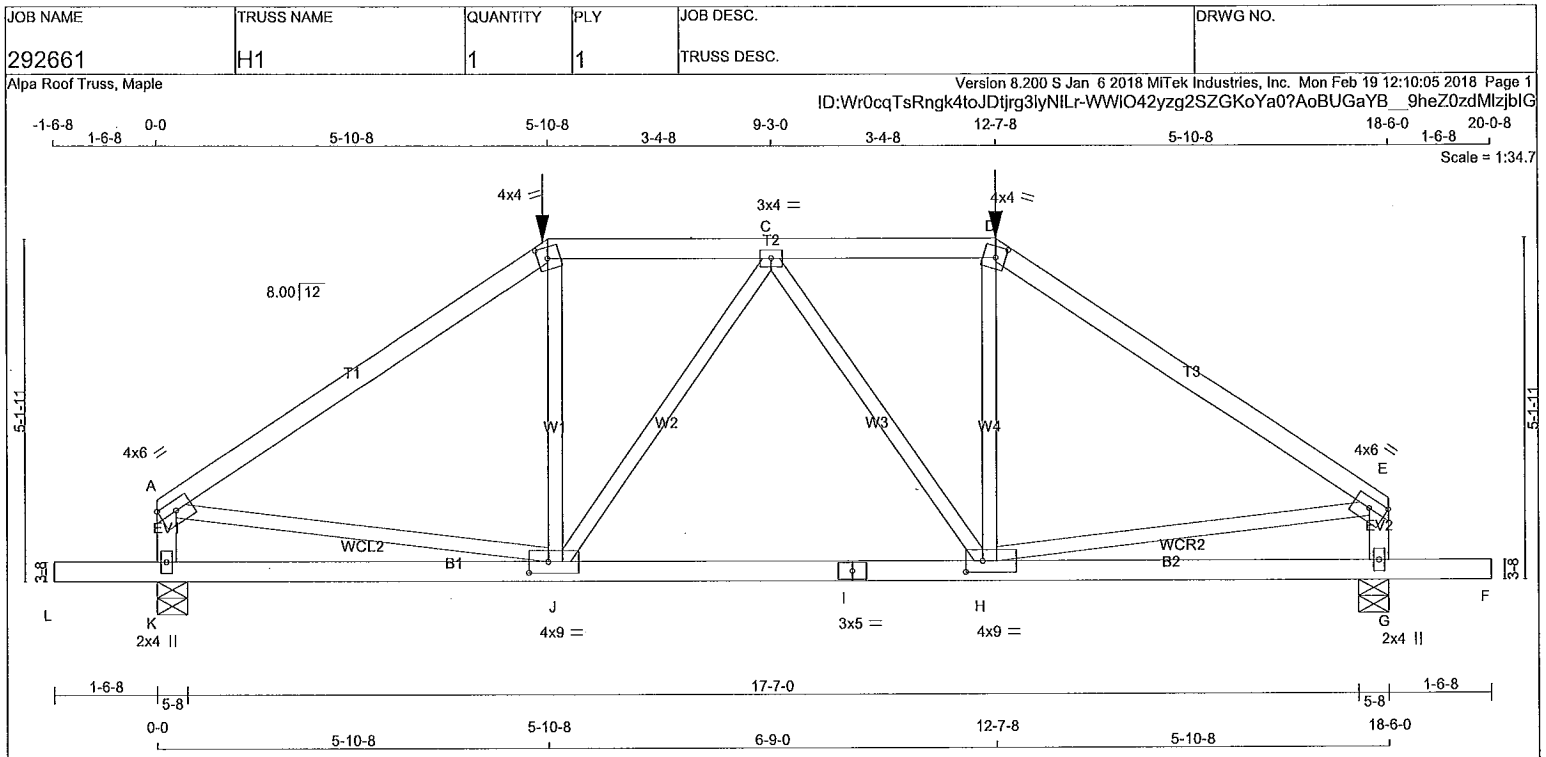
69 Graydon Crescent
Richmond Hill, Ontario
L4B 3W7
(905) 832-2250 Fax (905) 832-0286



1. Stracon Engineering Inc. is responsible for the design of trusses as individual components.
2. It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities with jurisdictions.
3. All dimensions are to be verified by owner contractor, architect or other authority before manufacture.
4. Stracon Engineering Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Stracon Engineering Inc. drawings is specified for the truss as a component only and forms an integral part of the truss design.
5. It is the truss manufacture's responsibility to ensure that trusses are manufactured in conformance with Stracon Engineering Inc. specifications outlined below.

1. Trusses designed by Stracon Engineering Inc. conforms to the relevant section of the Ontario Building Code of Canada (Part 9 or Part 4) or to the Canadian code for farm buildings, whichever applies to the building type indicated on the drawings as well as to the procedures established by the truss plate institute of Canada. Unit stresses used are as per CSA-086-09.
2. Lumber is to be the sizes and grade specified.
3. Moisture content of lumber is not to exceed 19% in service unless otherwise specified.
4. Lumber not to be treated with chemicals unless otherwise specified.
5. Plates shall be applied to both faces of the truss at each joint and shall be positioned as specified.
6. The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals not exceeding 12.5 times in thickness.
7. Where not rigid ceiling is attached directly to the bottom chord, laterally brace the chords at intervals not exceeding 3M (10') o.c.

January 15, 2014



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2 SPF
B - D	2x4	DRY	No.2 SPF
D - E	2x4	DRY	No.2 SPF
K - A	2x4	DRY	No.2 SPF
G - E	2x4	DRY	No.2 SPF
L - I	2x4	DRY	No.2 SPF
I - F	2x4	DRY	No.2 SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0	1.75	Edge
B	TTW-m	MT20	4.0	4.0	2.00	1.75
C	TMVW-t	MT20	3.0	4.0		
D	TTW-m	MT20	4.0	4.0	2.00	1.75
E	TMVW-t	MT20	4.0	6.0	1.75	Edge
G	BMV1+p	MT20	2.0	4.0		
H	BMVWW-t	MT20	4.0	9.0	2.00	3.00
I	BS-t	MT20	3.0	5.0		
J	BMVWW-t	MT20	4.0	9.0	2.00	3.50
K	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 334.0 lbs FACTORED DOWN AT 12-7-8, AND 334.0 lbs FACTORED DOWN AT 5-10-8 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ
JT	1796	0	5-8	2-11
K	1796	0	5-8	2-11

UNFACTORED REACTIONS

1ST CASE	MAX/MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	1283	770 / 0	0 / 0	0 / 0	0 / 0	512 / 0	0 / 0
G	1283	770 / 0	0 / 0	0 / 0	0 / 0	512 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.99 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX CSI (LC)	WEBS	MAX. FACTORED FORCE (LBS)	FACTORED MAX CSI (LC)
FR-TO		FROM	TO	LENGTH	FR-TO	
A-B	-1954 / 0	-78.0	-78.0	0.72 (1)	3.99	J-B 0 / 317
B-C	-1835 / 0	-153.5	-153.5	0.35 (1)	4.74	J-C -373 / 0
C-D	-1835 / 0	-153.5	-153.5	0.35 (1)	4.74	C-H -373 / 0
D-E	-1954 / 0	-78.0	-78.0	0.72 (1)	3.99	H-D 0 / 317
K-A	-1558 / 0	0.0	0.0	0.17 (1)	6.56	A-J 0 / 1642
G-E	-1558 / 0	0.0	0.0	0.17 (1)	6.56	H-E 0 / 1642

L-K	0 / 0	-96.5	-96.5	0.17 (1)	10.00	
K-J	0 / 0	-36.4	-36.4	0.35 (4)	10.00	
J-I	0 / 1841	-36.4	-36.4	0.54 (4)	10.00	
I-H	0 / 1841	-36.4	-36.4	0.54 (4)	10.00	
H-G	0 / 0	-36.4	-36.4	0.35 (4)	10.00	
G-F	0 / 0	-96.5	-96.5	0.17 (1)	10.00	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
B	5-10-8	-334	-334	---	FRONT	VERT	TOTAL
D	12-7-8	-334	-334	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 5-10-8
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADD'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.62")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.62")
CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL) = L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.72/1.00 (D-E:1), BC=0.54/1.00 (H-J:4), WB=0.41/1.00 (E-H:1), SSI=0.27/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

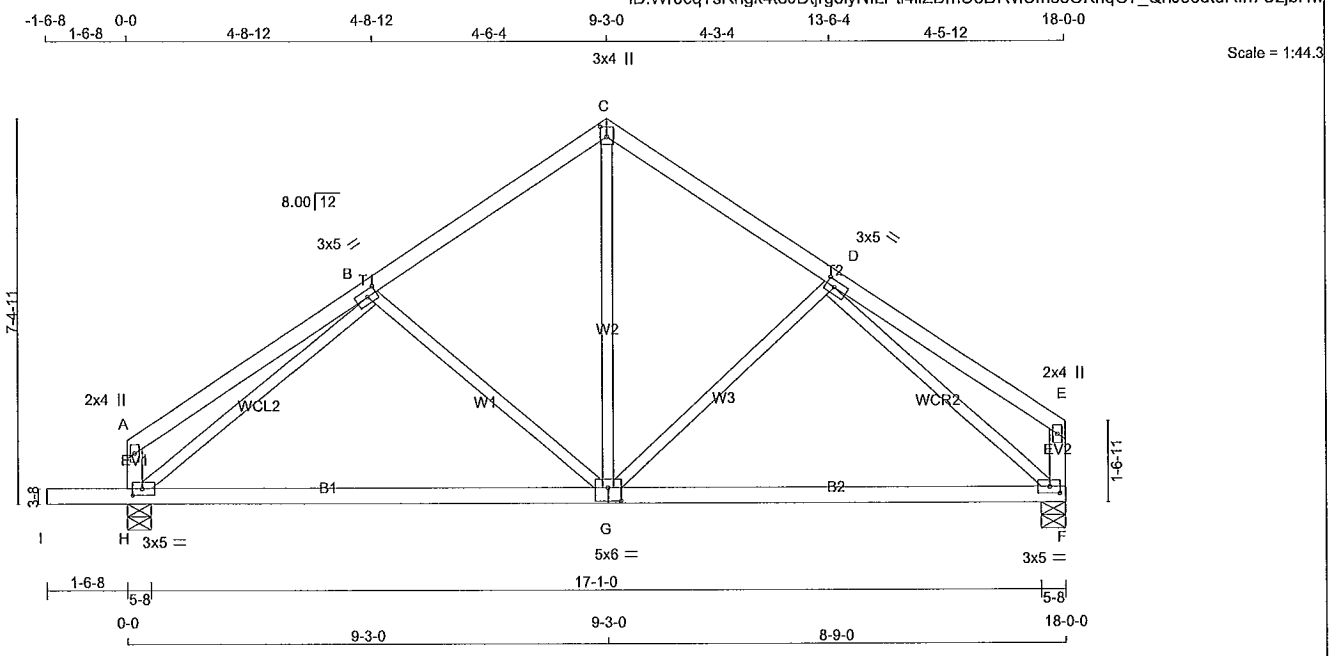
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

A-18023944 CONTINUED ON PAGE 2





TOTAL WEIGHT = 74 lb
[M][F]

LUMBER					
N, L, G, A, RULES	CHORDS	SIZE	LUMBER	DESCR.	
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
H - A	2x4	DRY	No.2	SPF	
F - E	2x4	DRY	No.2	SPF	
I - G	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMV+p	MT20	2.0	4.0	
B	TMVW-t	MT20	3.0	5.0	1.50 2.25
C	TTW+p	MT20	3.0	4.0	2.25 1.50
D	TMVW-t	MT20	3.0	5.0	1.50 2.00
E	TMV+p	MT20	2.0	4.0	
F	BMVW-t	MT20	3.0	5.0	1.50 2.25
G	BSWW-t	MT20	5.0	6.0	3.00 3.00
H	BMVW-t	MT20	3.0	5.0	1.50 2.25

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
H	1017	0	1017	0	0
F	868	0	868	0	0

UNFACTORED REACTIONS		1ST LCASE	MAX./MIN.	COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND
H	725	443 / 0	0 / 0	0 / 0	0 / 0
F	619	378 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX LC1 (LC)
FR-TO		FROM	TO		FR-TO		
A-B	0 / 24	-78.0	-78.0 0.27 (1)	10.00	B-G	-258 / 0	0.17 (1)
B-C	-687 / 0	-78.0	-78.0 0.21 (1)	6.25	G-C	0 / 467	0.11 (1)
C-D	-686 / 0	-78.0	-78.0 0.19 (1)	6.25	G-D	-192 / 0	0.12 (1)
D-E	0 / 22	-78.0	-78.0 0.24 (1)	10.00	H-B	-988 / 0	0.63 (1)
H-A	-138 / 0	0.0	0.0 0.01 (1)	7.81	D-F	-954 / 0	0.59 (1)
F-E	-130 / 0	0.0	0.0 0.01 (1)	7.81			
I-H	0 / 0	-96.5	-96.5 0.16 (1)	10.00			
H-G	0 / 748	-18.5	-18.5 0.52 (4)	10.00			
G-F	0 / 692	-18.5	-18.5 0.47 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.60")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.60")
CALCULATED VERT. DEFL.(TL) = 1/616 (0.35")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.01")

CSI: TC=0.27/1.00 (A-B:1), BC=0.52/1.00 (G-H:4), WB=0.63/1.00 (B-H:1), SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

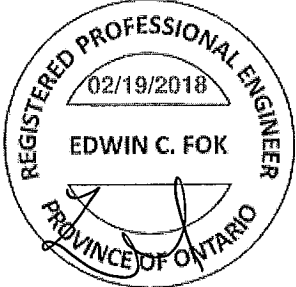
NAIL VALUES					
PLATE	GRIP(DRY)	SHEAR	SECTION		
(PSI)	(PLI)	(PLI)	(PLI)		
MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987

PLATE PLACEMENT TOL. = 0.250 inches

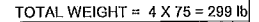
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.27 (B) (INPUT = 1.00)

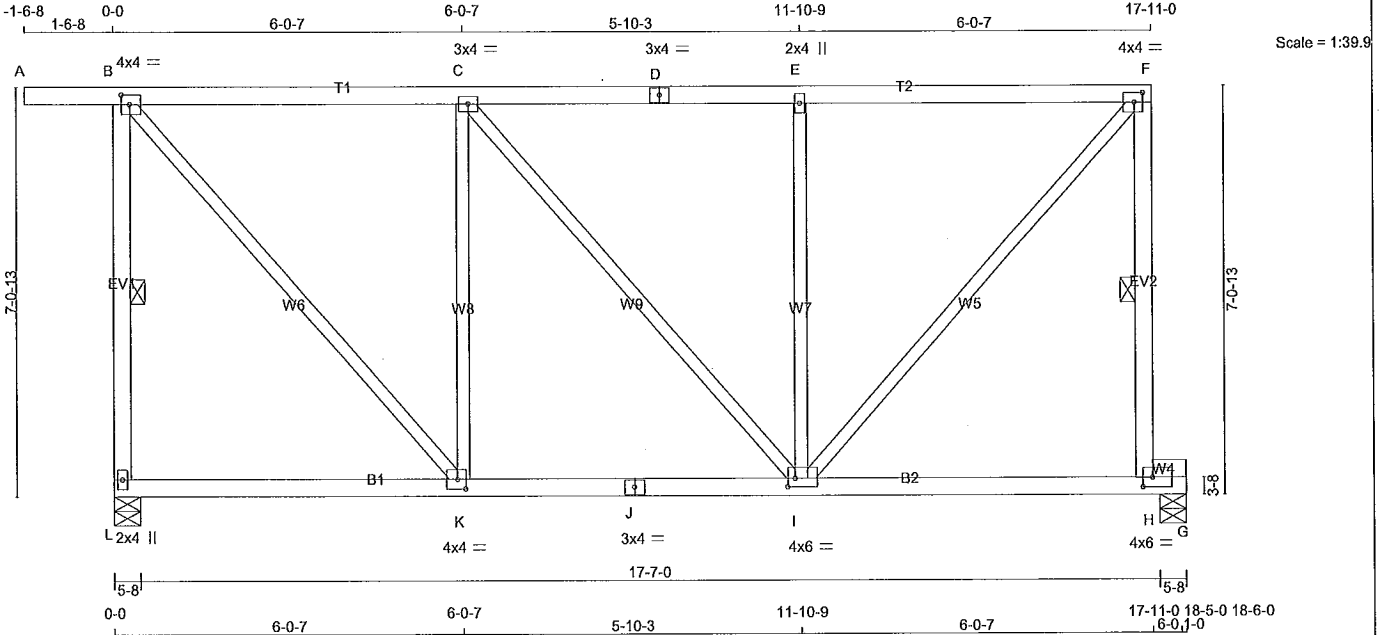
NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



Scale = 1:44.3



A-18023947



TOTAL WEIGHT = 90 lb (M)

LUMBER					
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.	
L - B	2x4	DRY	No.2	SPF	
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
L - J	2x4	DRY	No.2	SPF	
J - G	2x4	DRY	No.2	SPF	
H - G	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-I	MT20	4.0	4.0	2.00 1.75
C	TMWW-I	MT20	3.0	4.0	
D	TS-I	MT20	3.0	4.0	
E	TMVW+V	MT20	2.0	4.0	
F	TMVW-I	MT20	4.0	4.0	2.00 1.75
H	BMVW-I	MT20	4.0	6.0	2.00 2.00
I	BMWWW-I	MT20	4.0	6.0	1.75 1.50
J	BS-I	MT20	3.0	4.0	
K	BMWW-I	MT20	4.0	4.0	2.00 1.75
L	BMV1+p	MT20	2.0	4.0	

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT		VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
L	1012	0		1012	0	0	5-8	1-8	
G	848	0		848	0	0	5-8	1-8	

		UNFACTORED REACTIONS		MAX./MIN. COMPONENT REACTIONS					
JT		1ST LCASE	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	719	453 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	266 / 0	0 / 0
G	605	364 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	241 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF B-L, F-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX CSI (LC)
FR-TO		FROM	TO	FR-TO		FROM	TO
L-B	-961 / 0	0.0	0.0 0.22 (1)	I-F	0 / 941	0.21 (1)	
A-B	0 / 0	-78.0	-78.0 0.13 (1)	B-K	0 / 871	0.20 (1)	
B-C	-579 / 0	-78.0	-78.0 0.39 (1)	I-E	-507 / 0	0.44 (1)	
C-D	-626 / 0	-78.0	-78.0 0.39 (1)	K-C	-563 / 0	0.49 (1)	
D-E	-626 / 0	-78.0	-78.0 0.39 (1)	C-I	0 / 71	0.02 (1)	
E-F	-626 / 0	-78.0	-78.0 0.39 (1)				
H-F	-894 / 0	0.0	0.0 0.20 (1)				
L-K	0 / 0	-18.5	-18.5 0.14 (4)				
K-J	0 / 579	-18.5	-18.5 0.35 (1)				
J-I	0 / 579	-18.5	-18.5 0.35 (1)				
I-H	0 / 0	-18.5	-18.5 0.67 (1)				
H-G	0 / 0	-18.5	-18.5 0.28 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.62")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.62")
CALCULATED VERT. DEFL.(TL)= L/ 676 (0.33")

CSI: TC=0.39/1.00 (C-E:1), BC=0.67/1.00 (H-I:1), WB=0.49/1.00 (C-K:1), SSI=0.33/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

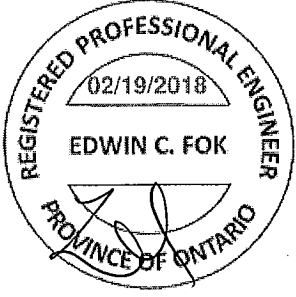
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES			
PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
MT20	618	354	1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

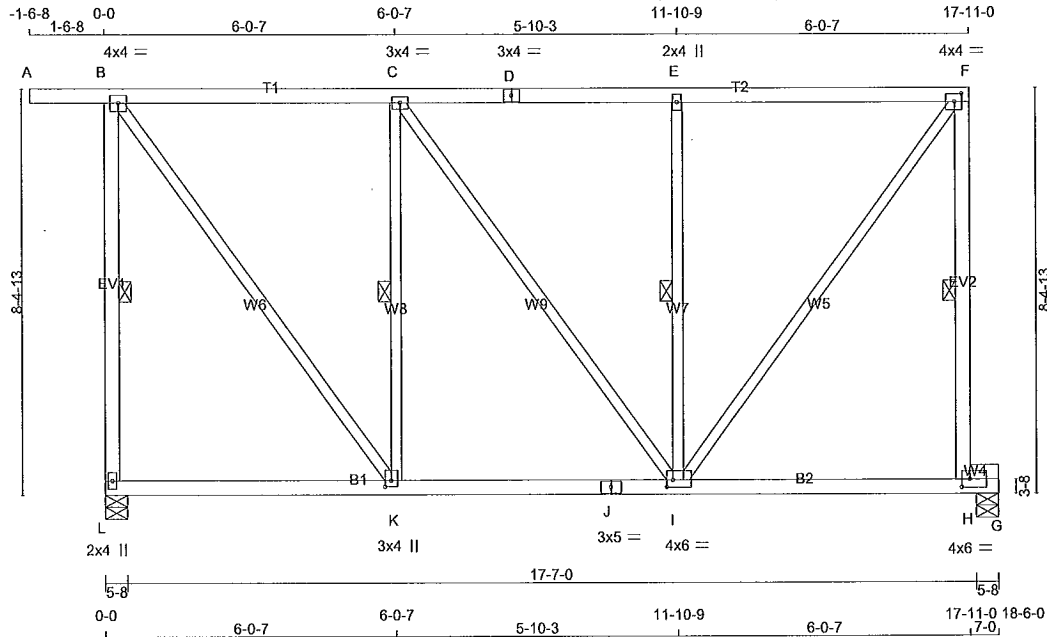
JSI GRIP= 0.85 (F) (INPUT = 0.90)
JSI METAL= 0.53 (H) (INPUT = 1.00)



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292661	H5	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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TOTAL WEIGHT = 2 X 98 = 196 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
L - B	2x4	DRY	No.2	SPF	
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
L - J	2x4	DRY	No.2	SPF	
J - G	2x4	DRY	No.2	SPF	
H - G	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0		
C	TMVW-t	MT20	3.0	4.0		
D	TS-t	MT20	3.0	4.0		
E	TMVW-t	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	4.0	2.00	1.75
H	BMVW-t	MT20	4.0	6.0	2.00	2.00
I	BMVW-t	MT20	4.0	6.0	1.75	1.50
J	BS-t	MT20	3.0	5.0		
K	BMVW-t	MT20	3.0	4.0	1.50	1.50
L	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

BEARINGS							
	FACTORED		MAXIMUM FACTORED			INPUT	REQD
	GROSS REACTION		GROSS REACTION			BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
L	1012	0	1012	0	0	5-8	1-8
G	848	0	848	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. SNOW	MAX/MIN. LIVE	MAX/MIN. PERM. LIVE	WIND	DEAD	SOIL
L	719	453 / 0	0 / 0	0 / 0	0 / 0	266 / 0	0 / 0
G	605	364 / 0	0 / 0	0 / 0	0 / 0	241 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF B-L, F-H, E-I, C-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
L-B	-961 / 0	0.0 0.0	0.31 (1)	6.25	I-F	0 / 876	0.20 (1)
A-B	0 / 0	-78.0 -78.0	0.13 (1)	10.00	B-K	0 / 811	0.18 (1)
B-C	-484 / 0	-78.0 -78.0	0.39 (1)	6.25	I-E	-506 / 0	0.23 (1)
C-D	-523 / 0	-78.0 -78.0	0.39 (1)	6.25	K-C	-563 / 0	0.25 (1)
D-E	-523 / 0	-78.0 -78.0	0.39 (1)	6.25	C-I	0 / 66	0.01 (1)
E-F	-523 / 0	-78.0 -78.0	0.38 (1)	6.25			
H-F	-894 / 0	0.0 0.0	0.29 (1)	6.25			
L-K	0 / 0	-18.5 -18.5	0.14 (4)	10.00			
K-J	0 / 484	-18.5 -18.5	0.33 (1)	10.00			
J-I	0 / 484	-18.5 -18.5	0.33 (1)	10.00			
I-H	0 / 0	-18.5 -18.5	0.67 (1)	10.00			
H-G	0 / 0	-18.5 -18.5	0.28 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.62")
CALCULATED VERT. DEFL.(LL) = L/999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.62")
CALCULATED VERT. DEFL.(TL) = L/677 (0.33")

CSI: TC=0.39/1.00 (C-E:1), BC=0.67/1.00 (H-I:1), WB=0.25/1.00 (C-K:1), SSI=0.33/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

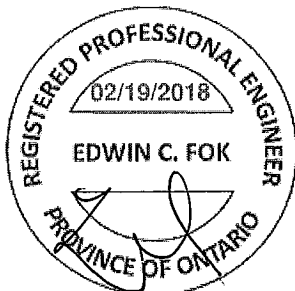
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

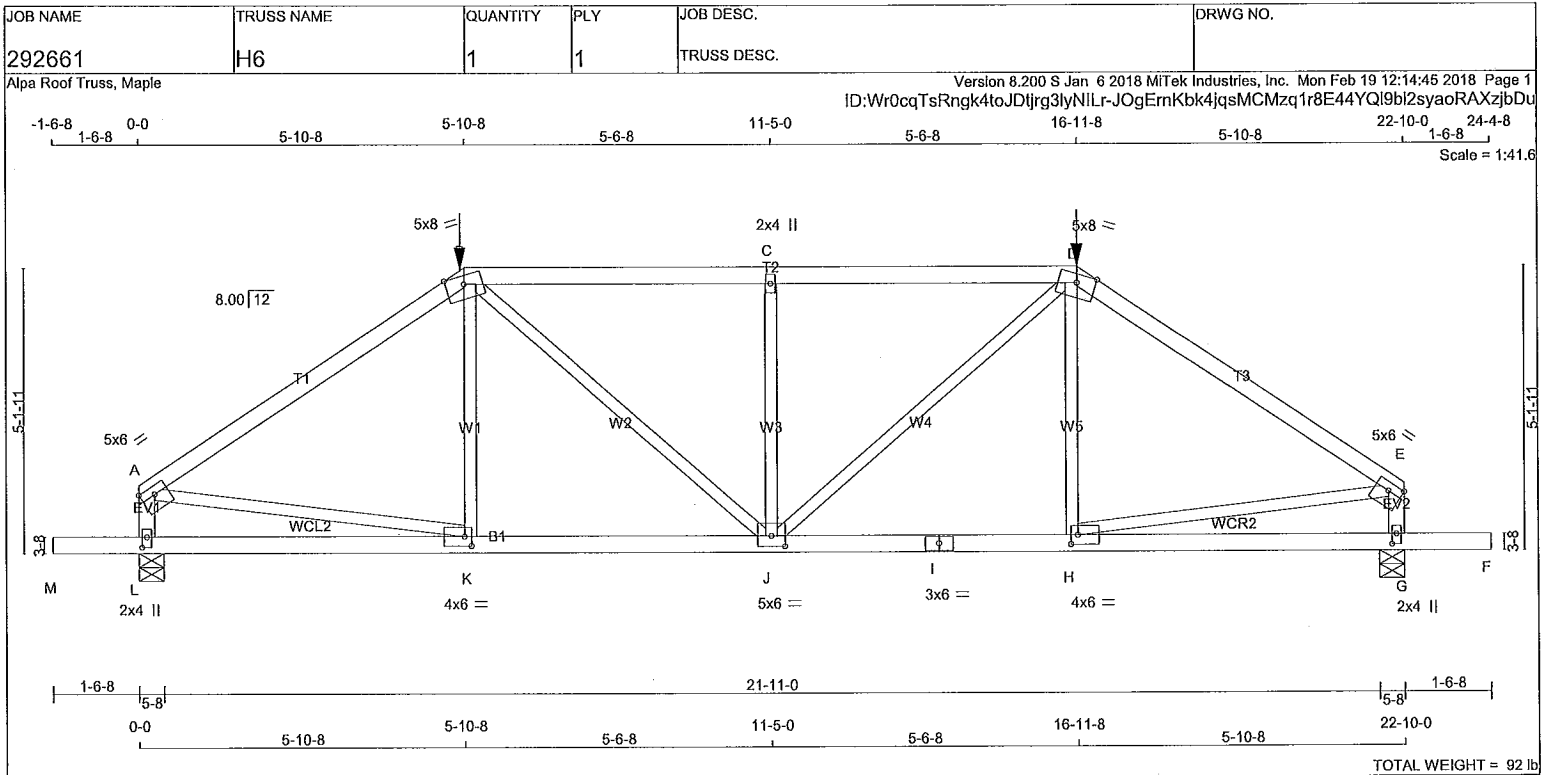
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (K) (INPUT = 0.90)
JSI METAL= 0.53 (H) (INPUT = 1.00)

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



A-18023949



LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	1850F 1.5E	SPF
D - E	2x4	DRY	No.2	SPF
L - A	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0	1.75	Edge
B	TTWW-m	MT20	5.0	8.0		Edge
C	TMVW-w	MT20	2.0	4.0		
D	TTWW-m	MT20	5.0	8.0		Edge
E	TMVW-t	MT20	5.0	6.0	1.75	Edge
G	BMV1+p	MT20	2.0	4.0	2.25	1.00
H	BMVW-t	MT20	4.0	6.0	2.00	1.50
I	BS-t	MT20	3.0	6.0		
J	BMVWW-t	MT20	5.0	6.0	2.25	3.00
K	BMVW-t	MT20	4.0	6.0	2.00	1.50
L	BMV1+p	MT20	2.0	4.0	2.25	1.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 334.0 lbs FACTORED DOWN AT 16-11-8, AND 334.0 lbs FACTORED DOWN AT 5-10-8 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
L	2207	0	2207	0
G	2207	0	2207	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN	COMPONENT REACTIONS	
L	1576	949 / 0	0 / 0	0 / 0
G	1576	949 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.36 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (C)	MAX UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX LC1 (C)
FR-TO		FROM TO			FR-TO		
A-B	-2554 / 0	-78.0 -78.0	0.83 (1)	3.47	K-B	-79 / 146	0.06 (4)
B-C	-2814 / 0	-153.5 -153.5	0.87 (1)	3.36	B-J	0 / 920	0.23 (1)
C-D	-2814 / 0	-153.5 -153.5	0.87 (1)	3.36	J-C	-1038 / 0	0.42 (1)
D-E	-2554 / 0	-78.0 -78.0	0.83 (1)	3.47	J-D	0 / 920	0.23 (1)
L-A	-1970 / 0	0.0 0.0	0.22 (1)	5.95	H-D	-79 / 146	0.06 (4)
G-E	-1970 / 0	0.0 0.0	0.22 (1)	5.95	A-K	0 / 2147	0.53 (1)
					H-E	0 / 2147	0.53 (1)
M-L	0 / 0	-96.5 -96.5	0.17 (1)	10.00			
L-K	0 / 0	-36.4 -36.4	0.33 (4)	10.00			
K-J	0 / 2121	-36.4 -36.4	0.56 (1)	10.00			
J-I	0 / 2121	-36.4 -36.4	0.56 (1)	10.00			
I-H	0 / 2121	-36.4 -36.4	0.56 (1)	10.00			
H-G	0 / 0	-36.4 -36.4	0.33 (4)	10.00			
G-F	0 / 0	-96.5 -96.5	0.17 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
B	5-10-8	-334	-334	---	FRONT	VERT	TOTAL
D	16-11-8	-334	-334	---	FRONT	VERT	TOTAL

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 21.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 5-10-8
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADD'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.76")
CALCULATED VERT. DEFL.(LL)= L/999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.76")
CALCULATED VERT. DEFL.(TL)= L/999 (0.16")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL)= L/999 (0.01")

CSI: TC=0.87/1.00 (C-D:1), BC=0.56/1.00 (J-K:1), WB=0.53/1.00 (A-K:1), SSI=0.45/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

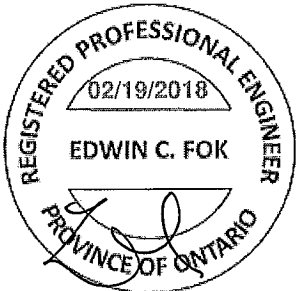
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

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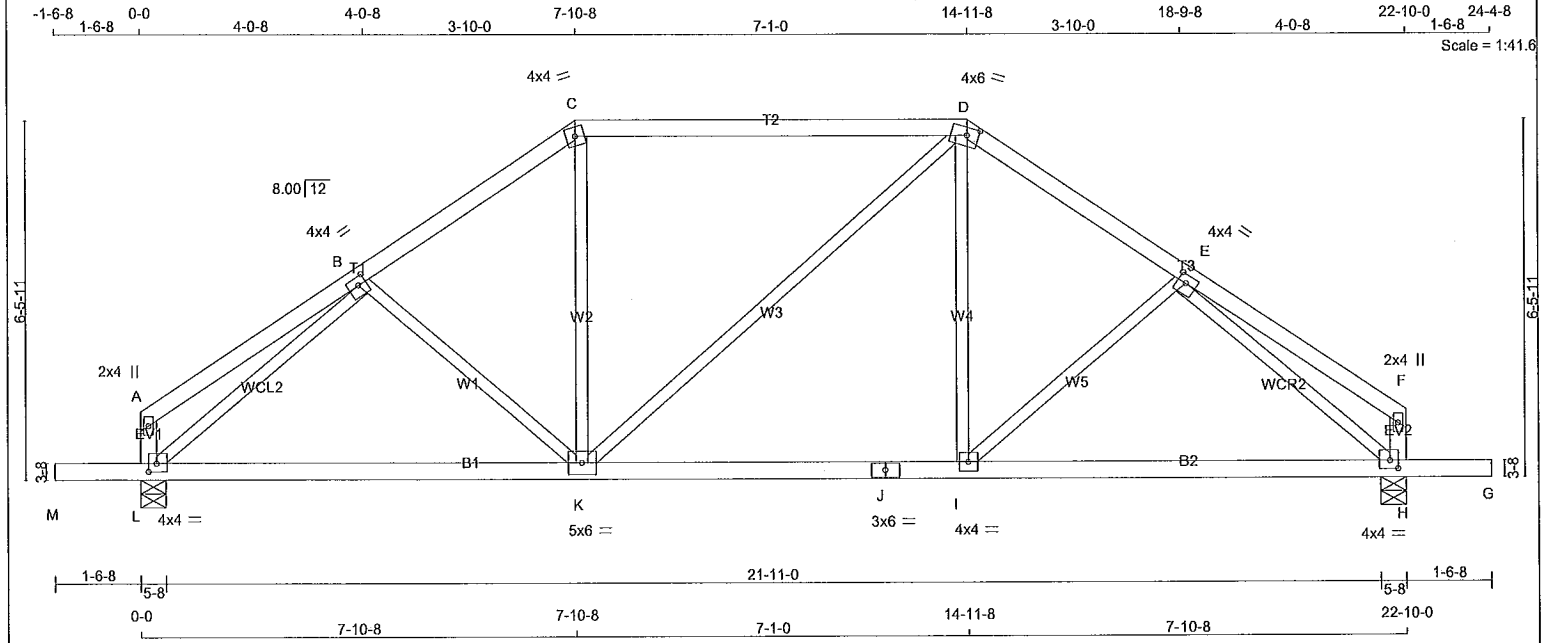
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292661	H7	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	2100F 1.8E	SPF	
D - F	2x4	DRY	No.2	SPF	
L - A	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
M - J	2x4	DRY	No.2	SPF	
J - G	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0		
B	TMVW-t	MT20	4.0	4.0	1.75	1.75
C	TTW-m	MT20	4.0	4.0		
D	TTWW-m	MT20	4.0	6.0	1.75	2.50
E	TMVW-t	MT20	4.0	4.0	1.75	1.75
F	TMV+p	MT20	2.0	4.0		
H	BMVW-t	MT20	4.0	4.0	1.75	1.75
I	BMVW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMVW-t	MT20	5.0	6.0		
L	BMVW-t	MT20	4.0	4.0	1.75	1.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
L	1250	0	1250	0	5-8	1-8
H	1250	0	1250	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	891	544 / 0	0 / 0	0 / 0	0 / 0	347 / 0	0 / 0
H	891	544 / 0	0 / 0	0 / 0	0 / 0	347 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.78 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		MAX. UNBRACED LENGTH	MEMB.	WEBS	
	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LBS)	LC2 MAX (LBS)			MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO						FR-TO		
A-B	0 / 19	-78.0	-78.0	0.18 (1)	10.00	B-K	-123 / 4	0.06 (1)
B-C	-1144 / 0	-78.0	-78.0	0.16 (1)	5.78	K-C	0 / 231	0.06 (4)
C-D	-939 / 0	-78.0	-78.0	0.34 (1)	6.25	K-D	0 / 0	0.00 (1)
D-E	-1144 / 0	-78.0	-78.0	0.16 (1)	5.78	I-D	0 / 230	0.06 (4)
E-F	0 / 19	-78.0	-78.0	0.18 (1)	10.00	I-E	-123 / 4	0.06 (1)
L-A	-119 / 0	0.0	0.0	0.01 (1)	7.81	L-B	-1380 / 0	0.62 (1)
H-F	-119 / 0	0.0	0.0	0.01 (1)	7.81	E-H	-1379 / 0	0.62 (1)
M-L	0 / 0	-96.5	-96.5	0.16 (1)	10.00			
L-K	0 / 1027	-18.5	-18.5	0.33 (4)	10.00			
K-J	0 / 938	-18.5	-18.5	0.34 (4)	10.00			
J-I	0 / 938	-18.5	-18.5	0.34 (4)	10.00			
I-H	0 / 1027	-18.5	-18.5	0.33 (4)	10.00			
H-G	0 / 0	-96.5	-96.5	0.16 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.76")
CALCULATED VERT. DEFL.(LL)= L/999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.76")
CALCULATED VERT. DEFL.(TL)= L/999 (0.14")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL)= L/999 (0.01")

CSI: TC=0.34/1.00 (C-D:1), BC=0.34/1.00 (I-K:4), WB=0.62/1.00 (B-L:1), SSI=0.22/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (L) (INPUT = 0.90)
JSI METAL= 0.50 (B) (INPUT = 1.00)

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

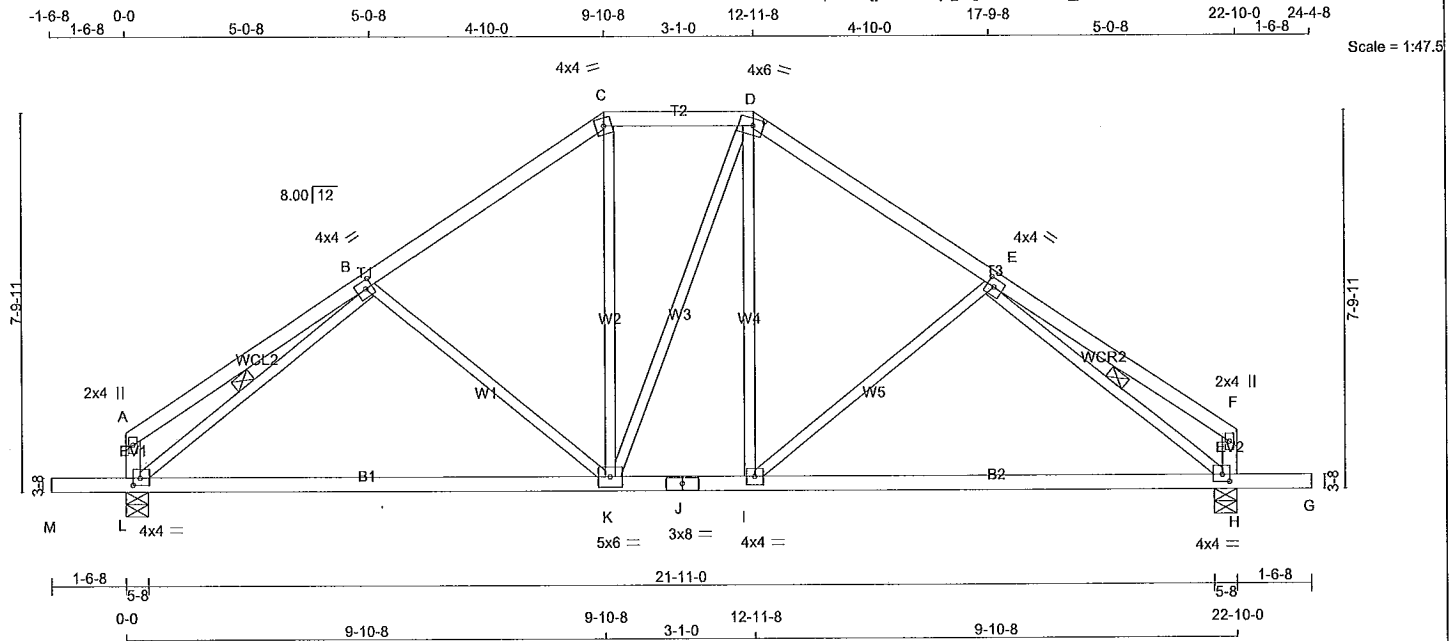


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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292661	H8	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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TOTAL WEIGHT = 101 lb
[M][F]

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
L - A	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
M - J	2x4	DRY	No.2	SPF	
J - G	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0			
B	TMWW-t	MT20	4.0	4.0	2.00	1.75	
C	TTW-m	MT20	4.0	4.0			
D	TTWW-m	MT20	4.0	6.0	1.75	2.50	
E	TMWW-t	MT20	4.0	4.0	2.00	1.75	
F	TMV+p	MT20	2.0	4.0			
H	BMVW1-t	MT20	4.0	4.0	1.75	1.75	
I	BMWW-t	MT20	4.0	4.0			
J	BS-t	MT20	3.0	8.0			
K	BMWWW-t	MT20	5.0	6.0			
L	BMVW1-t	MT20	4.0	4.0	1.75	1.75	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
L	1250	0	1250	0	0	5-8	1-8
H	1250	0	1250	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN	COMPONENT REACTIONS	WIND	DEAD	SOIL
L	891	544 / 0	0 / 0	0 / 0	347 / 0	0 / 0
H	891	544 / 0	0 / 0	0 / 0	347 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.87 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF B-L, E-H.

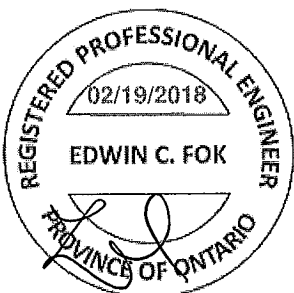
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MAX. FACTORED		FACTORED		MAX. FACTORED		MAX. FACTORED		
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM TO			FR-TO			
A-B	0 / 25	-78.0	-78.0	0.31 (1)	10.00	B-K	-250 / 0	0.20 (1)
B-C	-1037 / 0	-78.0	-78.0	0.25 (1)	5.87	K-C	0 / 295	0.07 (1)
C-D	-845 / 0	-78.0	-78.0	0.10 (1)	6.25	K-D	0 / 1	0.00 (1)
D-E	-1036 / 0	-78.0	-78.0	0.25 (1)	5.88	I-D	0 / 293	0.07 (1)
E-F	0 / 25	-78.0	-78.0	0.31 (1)	10.00	I-E	-251 / 0	0.20 (1)
L-A	-147 / 0	0.0	0.0	0.01 (1)	7.81	L-B	-1356 / 0	0.39 (1)
H-F	-147 / 0	0.0	0.0	0.01 (1)	7.81	E-H	-1355 / 0	0.39 (1)
M-L	0 / 0	-96.5	-96.5	0.16 (1)	10.00			
L-K	0 / 1033	-18.5	-18.5	0.47 (4)	10.00			
K-J	0 / 844	-18.5	-18.5	0.47 (4)	10.00			
J-I	0 / 844	-18.5	-18.5	0.47 (4)	10.00			
I-H	0 / 1033	-18.5	-18.5	0.47 (4)	10.00			
H-G	0 / 0	-96.5	-96.5	0.16 (1)	10.00			

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.76")
CALCULATED VERT. DEFL.(LL)= L/999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.76")
CALCULATED VERT. DEFL.(TL)= L/910 (0.30")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL)= L/999 (0.01")

CSI: TC=0.31/1.00 (E-F:1), BC=0.47/1.00 (K-L:4), WB=0.39/1.00 (B-L:1), SSI=0.15/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

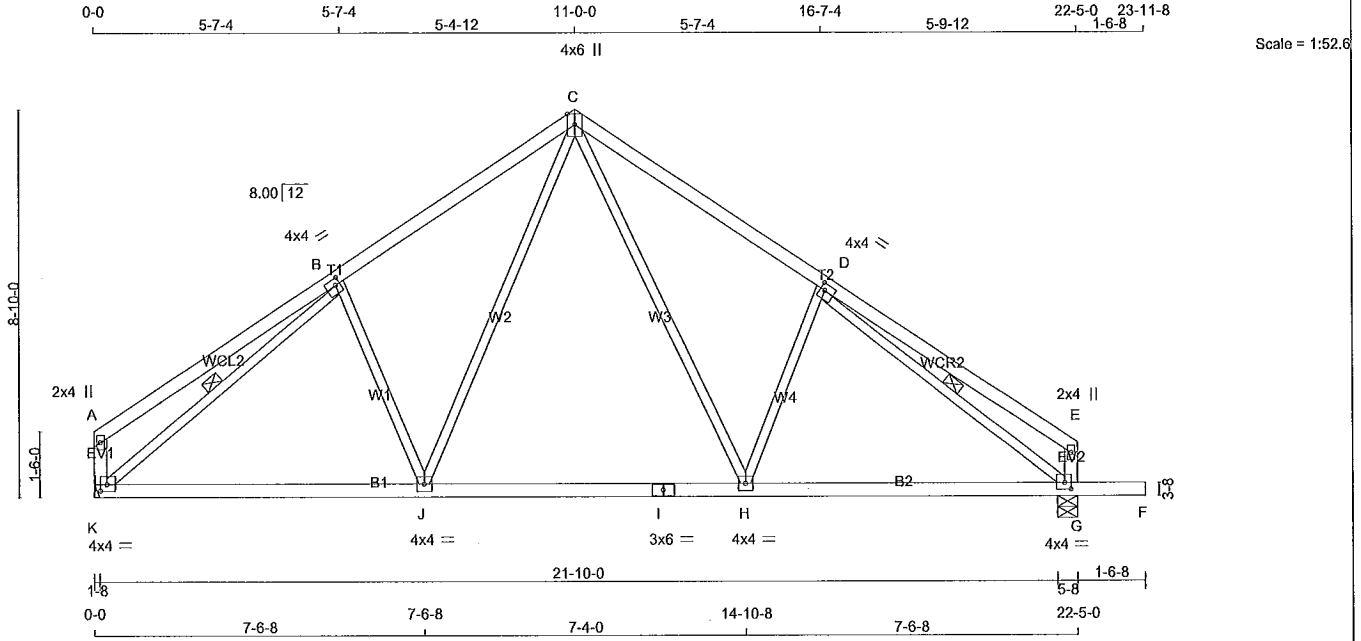
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.81 (J) (INPUT = 1.00)

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LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0		
B	TMWW-t	MT20	4.0	4.0	1.75	1.25
C	TTWW+p	MT20	4.0	6.0	Edge	
D	TMWW-t	MT20	4.0	4.0	1.75	1.25
E	TMV+p	MT20	2.0	4.0		
G	BMVW1-t	MT20	4.0	4.0	1.75	1.75
H	BMWW-t	MT20	4.0	4.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	4.0	1.75	1.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
K	1081	0	1081	0	0	0
G	1230	0	1230	0	0	0

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	771	471 / 0	0 / 0	0 / 0	0 / 0	300 / 0	0 / 0
G	877	535 / 0	0 / 0	0 / 0	0 / 0	342 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.56 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF B-K, D-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRAC LENGTH
FR-TO					FR-TO		
A-B	0 / 28	-78.0	-78.0 0.39 (1)	10.00	B-J	-271 / 0	0.13 (1)
B-C	-1074 / 0	-78.0	-78.0 0.32 (1)	5.71	J-C	0 / 432	0.10 (1)
C-D	-1130 / 0	-78.0	-78.0 0.35 (1)	5.56	C-H	0 / 501	0.11 (1)
D-E	0 / 30	-78.0	-78.0 0.42 (1)	10.00	H-D	-329 / 0	0.14 (1)
K-A	-182 / 0	0.0	0.0 0.02 (1)	7.81	K-B	-1303 / 0	0.46 (1)
G-E	-168 / 0	0.0	0.0 0.02 (1)	7.81	D-G	-1344 / 0	0.48 (1)
K-J	0 / 977	-18.5	-18.5 0.33 (4)	10.00			
J-I	0 / 708	-18.5	-18.5 0.30 (4)	10.00			
I-H	0 / 708	-18.5	-18.5 0.30 (4)	10.00			
H-G	0 / 1036	-18.5	-18.5 0.33 (4)	10.00			
G-F	0 / 0	-96.5	-96.5 0.16 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= 1/360 (0.75")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.04")
ALLOWABLE DEFL.(TL)= 1/360 (0.75")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.12")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= 1/120 (0.19")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.01")
ALLOWABLE DEFL.(TL)= 1/120 (0.19")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.01")

CSI: TC=0.42/1.00 (D-E:1) , BC=0.33/1.00 (G-H:4) , WB=0.48/1.00 (D-G:1) , SSI=0.18/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 L.S BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

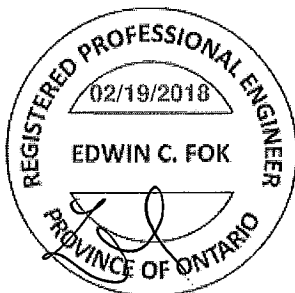
PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (G) (INPUT = 0.90)
JSI METAL= 0.53 (D) (INPUT = 1.00)

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



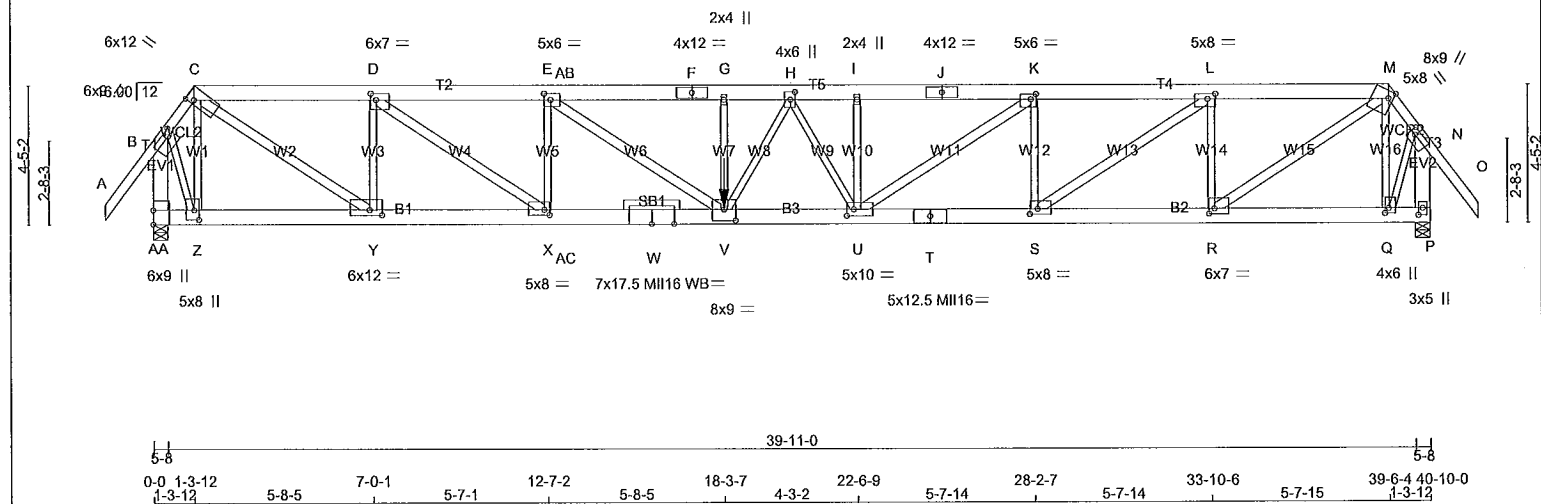
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292661	H10A	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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1-6-8 0-0 1-3-12 5-8-5 7-0-1 5-7-1 12-7-2 5-8-5 18-3-7 20-5-0 22-6-9 5-7-14 28-2-7 5-7-14 33-10-6 5-7-15 39-6-4 40-10-0 42-4-8
1-6-8 1-3-12 5-8-5 7-0-1 5-7-1 12-7-2 5-8-5 18-3-7 2-1-9 2-1-9 5-7-14 28-2-7 5-7-14 33-10-6 5-7-15 39-6-4 40-10-0 42-4-8
Scale = 1:73.8



LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES				
A - C	2x4	DRY	No.2	SPF
C - F	2x6	DRY	1650F 1.5E	SPF
F - J	2x6	DRY	1650F 1.5E	SPF
J - M	2x6	DRY	1650F 1.5E	SPF
M - O	2x4	DRY	No.2	SPF
AA - B	2x6	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
AA - W	2x6	DRY	2100F 1.8E	SPF
W - T	2x6	DRY	2100F 1.8E	SPF
T - P	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - Y	2x4	DRY	1650F 1.5E	SPF
D - X	2x4	DRY	1650F 1.5E	SPF
E - V	2x4	DRY	No.2	SPF
U - K	2x4	DRY	No.2	SPF
S - L	2x4	DRY	No.2	SPF
R - M	2x4	DRY	1650F 1.5E	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	6.0	9.0	2.00	2.50
C	TTWW-H	MT20	6.0	12.0	3.00	1.50
D	TMVW-t	MT20	6.0	7.0	2.50	2.00
E	TMVW-t	MT20	5.0	6.0	2.50	2.50
F	TS-t	MT20	4.0	12.0		
G	TMVW-w	MT20	2.0	4.0		
H	TMVW-H	MT20	4.0	6.0	3.00	1.75
I	TMVW-w	MT20	2.0	4.0		
J	TS-t	MT20	4.0	12.0		
K	TMVW-t	MT20	5.0	6.0	2.00	2.00
L	TMVW-t	MT20	5.0	8.0	2.00	3.00
M	TTWW+m	MT20	8.0	9.0	2.75	1.75
N	TMVW-t	MT20	5.0	8.0	2.00	2.00
P	BMV1+p	MT20	3.0	5.0	2.75	1.50
Q	BMVW-H	MT20	4.0	6.0	2.00	1.50
R	BMVW-t	MT20	6.0	7.0	2.00	2.00
S	BMVW-t	MT20	5.0	8.0	2.00	3.00

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
AA	5300	0	5300	0
P	3642	0	3642	0

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
AA	3773	2338 / 0	0 / 0	0 / 0	0 / 0	1435 / 0	0 / 0
P	2592	1609 / 0	0 / 0	0 / 0	0 / 0	982 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) AA, P
BEARING SIZE FACTOR = 1.15 AT JNT(S) AA, P (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	MAX.	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)
FR-TO		FROM TO		FR-TO	
A-B	0 / 52	-78.0 -78.0 0.17 (1)	10.00	Z-C	-1908 / 0
B-C	-3092 / 0	-78.0 -78.0 0.28 (1)	3.68	C-Y	0 / 7270
C-D	-7727 / 0	-78.0 -78.0 0.34 (1)	3.48	Y-D	-3291 / 0
D-E	-11734 / 0	-78.0 -78.0 0.55 (1)	2.73	D-X	0 / 4915
E-AB	-13716 / 0	-78.0 -78.0 0.86 (1)	2.25	X-E	-1963 / 0
AB-F	-13716 / 0	-118.5 -118.5 0.86 (1)	2.25	E-V	0 / 2415
F-G	-13716 / 0	-118.5 -118.5 0.86 (1)	2.25	V-G	-409 / 0
G-H	-13716 / 0	-78.0 -78.0 0.59 (1)	2.46	H-U	-1810 / 0
H-I	-12059 / 0	-78.0 -78.0 0.46 (1)	2.73	I-U	-327 / 0
I-J	-12059 / 0	-78.0 -78.0 0.62 (1)	2.64	U-I	-327 / 0
J-K	-12059 / 0	-78.0 -78.0 0.62 (1)	2.64	U-K	0 / 3462
K-L	-9225 / 0	-78.0 -78.0 0.39 (1)	3.18	S-K	-2430 / 0
L-M	-5590 / 0	-78.0 -78.0 0.22 (1)	4.10	S-L	0 / 4441
M-N	-2068 / 0	-78.0 -78.0 0.22 (1)	4.47	R-L	-3037 / 0
N-O	0 / 52	-78.0 -78.0 0.17 (1)	10.00	R-M	0 / 5414
AA-B	-5517 / 0	0.0 0.0 0.55 (1)	4.53	Q-M	-1663 / 0
P-N	-3740 / 0	0.0 0.0 0.38 (1)	5.48	B-Z	0 / 3360
				Q-N	0 / 2224
AA-Z	0 / 0	-160.2 -160.2 0.19 (1)	10.00		
Z-Y	0 / 1761	-160.2 -160.2 0.28 (1)	10.00		
Y-X	0 / 7727	-160.2 -160.2 0.54 (1)	10.00		
X-AC	0 / 11734	-160.2 -160.2 0.79 (1)	10.00		
AC-W	0 / 11734	-124.0 -124.0 0.79 (1)	10.00		
W-V	0 / 11734	-124.0 -124.0 0.79 (1)	10.00		
V-U	0 / 12915	-18.5 -18.5 0.74 (1)	10.00		
U-T	0 / 9225	-18.5 -18.5 0.55 (1)	10.00		
T-S	0 / 9225	-18.5 -18.5 0.55 (1)	10.00		
S-R	0 / 5590	-18.5 -18.5 0.34 (1)	10.00		
R-Q	0 / 1155	-18.5 -18.5 0.12 (1)	10.00		
Q-P	0 / 0	-18.5 -18.5 0.06 (1)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
V	18-3-8	-2131	-2131	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL = 21.0	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 34.4	PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CSdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 7-10-8
END DISTANCE = 13-0-0
END SPAN CARRIED = 7-10-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDTL LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.36")
CALCULATED VERT. DEFL.(LL)= 1/801 (0.61")
ALLOWABLE DEFL.(TL)= L/360 (1.36")
CALCULATED VERT. DEFL.(TL)= 1/411 (1.19")

CSI: TC=0.86/1.00 (E-G:1) , BC=0.79/1.00 (V-X:1)
WB=0.93/1.00 (C-Y:1) , SS=0.32/1.00 (Y-Z:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

A-18023954

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292661	H10A	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Feb 19 12:19:59 2018 Page 2
ID:Wr0cqTsRngk4toJDtjrg3lyNlR-7oexl98ZnYHHXcCiNmftTs4kO?s2AgY49Qs2CKzjb9

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
T	BS-t	MII16	5.0	12.5		
U	BMWWW-t	MT20	5.0	10.0	2.50	2.75
V	BMWWW-t	MT20	8.0	9.0	4.25	4.50
W	BS-t	MII16	7.0	17.5		
X	BMWWW-t	MT20	5.0	8.0	2.00	2.00
Y	BMWWW-t	MT20	6.0	12.0	2.00	4.75
Z	BMWWW-t	MT20	5.0	8.0	3.75	1.75
AA	BMV1-t	MT20	6.0	9.0	5.50	

WB - INDICATES BLOCKING REQUIRED

HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 2130.5 lbs FACTORED DOWN AT 18-3-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NAIL VALUES

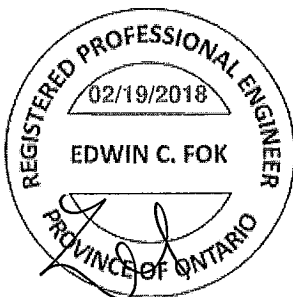
PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	618	354	1667
MII16	438	276	2341

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (L) (INPUT = 0.90)
JSI METAL= 0.95 (R) (INPUT = 1.00)

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



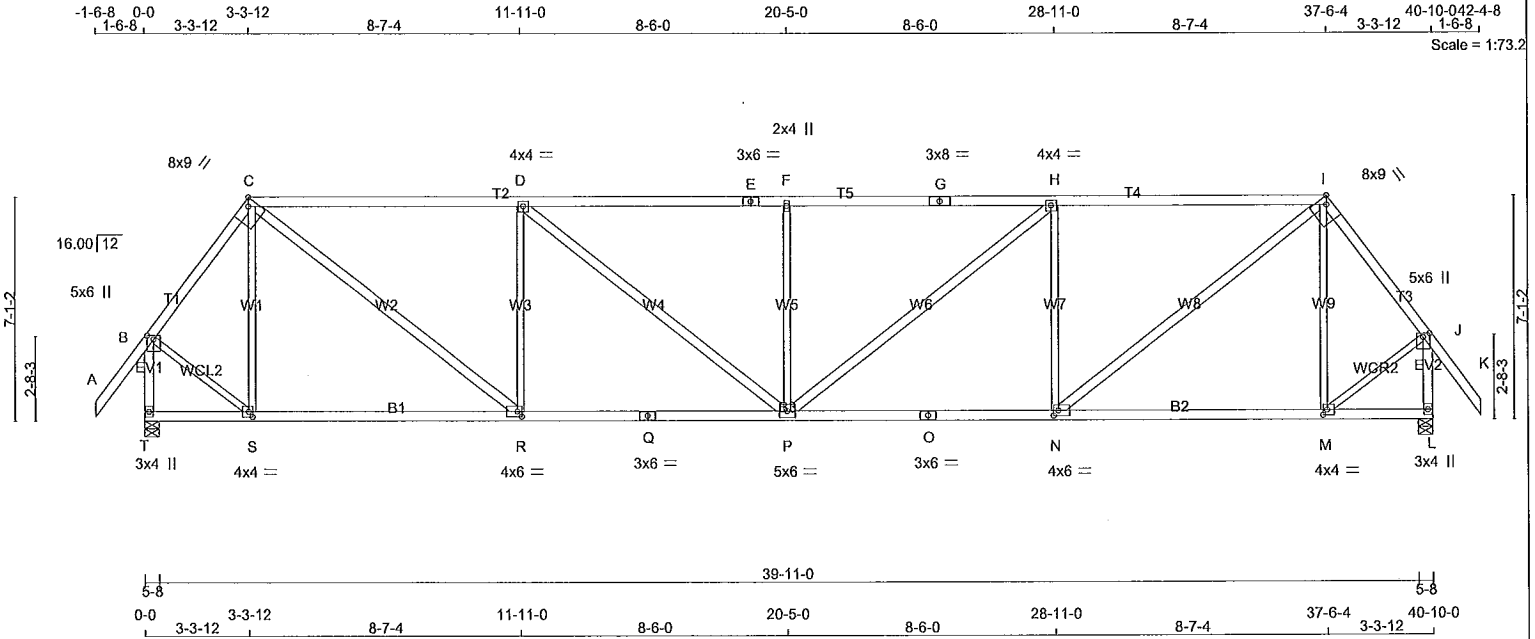
A-18023954(2)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292661	H12	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 8.200 S Jan 6 2018 MITek Industries, Inc. Mon Feb 19 12:20:23 2018 Page 1

ID:Wf0cqTsRngk4toJDfjrg3lyNfLr-QP50nMQc7wJtP12Jnp55rV4rEg6BnWdcg9gK2xzjb8c



TOTAL WEIGHT = 191 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY No.2	SPF
C - E	2x4	DRY 2100F 1.8E	SPF
E - G	2x4	DRY 2100F 1.8E	SPF
G - I	2x4	DRY 2100F 1.8E	SPF
I - K	2x4	DRY No.2	SPF
T - B	2x4	DRY No.2	SPF
L - J	2x4	DRY No.2	SPF
T - Q	2x4	DRY No.2	SPF
Q - O	2x4	DRY No.2	SPF
O - L	2x4	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF
C - R	2x4	DRY No.2	SPF
D - P	2x4	DRY No.2	SPF
P - H	2x4	DRY No.2	SPF
N - I	2x4	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMVW+p	MT20	5.0	6.0 1.50 2.50
C	TTWW-h	MT20	8.0	9.0 Edge 2.75
D	TMWW-t	MT20	4.0	4.0
E	TS-t	MT20	3.0	6.0
F	TMW+w	MT20	2.0	4.0
G	TS-t	MT20	3.0	8.0
H	TMWW-t	MT20	4.0	4.0
I	TTWW-h	MT20	8.0	9.0 Edge 2.75
J	TMVW+p	MT20	5.0	6.0 1.50 2.50
L	BMV1+p	MT20	3.0	4.0
M	BMWW-t	MT20	4.0	4.0 2.00 1.50
N	BMWW-t	MT20	4.0	6.0 2.00 1.75
O	BS-t	MT20	3.0	6.0
P	BMWW-t	MT20	5.0	6.0
Q	BS-t	MT20	3.0	6.0
R	BMWW-t	MT20	4.0	6.0 2.00 1.75
S	BMWW-t	MT20	4.0	4.0 2.00 1.50
T	BMV1+p	MT20	3.0	4.0

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
T	2099	0	2099	0	0	5-8	3-5
L	2099	0	2099	0	0	5-8	3-5

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1494	927 / 0	0 / 0	0 / 0	0 / 0	567 / 0	0 / 0
L	1494	927 / 0	0 / 0	0 / 0	0 / 0	567 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.03 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 MAX (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LC1 MAX (LBS)	MAX. FACTORED LC1 MAX (LBS)
FR-TO		FROM TO		FR-TO		FROM TO	
A-B	0 / 52	-78.0 -78.0	0.15 (1)	10.00	S-C	-477 / 0	0.42 (1)
B-C	-1525 / 0	-78.0 -78.0	0.20 (1)	5.14	C-R	0 / 2093	0.34 (1)
C-D	-2547 / 0	-78.0 -78.0	0.80 (1)	4.35	R-D	-1131 / 0	0.99 (1)
D-E	-3027 / 0	-78.0 -78.0	0.87 (1)	4.03	D-P	0 / 614	0.10 (1)
E-F	-3027 / 0	-78.0 -78.0	0.87 (1)	4.03	P-F	-612 / 0	0.53 (1)
F-G	-3027 / 0	-78.0 -78.0	0.87 (1)	4.03	P-H	0 / 614	0.10 (1)
G-H	-3027 / 0	-78.0 -78.0	0.87 (1)	4.03	N-H	-1131 / 0	0.99 (1)
H-I	-2547 / 0	-78.0 -78.0	0.80 (1)	4.35	N-I	0 / 2093	0.34 (1)
I-J	-1525 / 0	-78.0 -78.0	0.20 (1)	5.14	M-I	-477 / 0	0.42 (1)
J-K	0 / 52	-78.0 -78.0	0.15 (1)	10.00	B-S	0 / 1094	0.25 (1)
T-B	-2093 / 0	0.0 0.0	0.30 (1)	5.85	M-J	0 / 1094	0.25 (1)
L-J	-2093 / 0	0.0 0.0	0.30 (1)	5.85			
T-S	0 / 0	-18.5 -18.5	0.20 (4)	10.00			
S-R	0 / 901	-18.5 -18.5	0.36 (4)	10.00			
R-Q	0 / 2547	-18.5 -18.5	0.56 (1)	10.00			
Q-P	0 / 2547	-18.5 -18.5	0.56 (1)	10.00			
P-O	0 / 2547	-18.5 -18.5	0.56 (1)	10.00			
O-N	0 / 2547	-18.5 -18.5	0.56 (1)	10.00			
N-M	0 / 901	-18.5 -18.5	0.36 (4)	10.00			
M-L	0 / 0	-18.5 -18.5	0.20 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 21.0	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(65 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.36")
CALCULATED VERT. DEFL.(LL)= L/999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (1.36")
CALCULATED VERT. DEFL.(TL)= L/999 (0.32")

CSI: TC=0.87/1.00 (D-F:1), BC=0.56/1.00 (P-R:1), WB=0.99/1.00 (H-N:1), SSI=0.32/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES				
PLATE	GRIP(DRY)	SHEAR	SECTION	
	(PSI)	(PLI)	(PLI)	
	MAX MIN	MAX MIN	MAX MIN	
MT20	618	354	1667	788 1987 1656

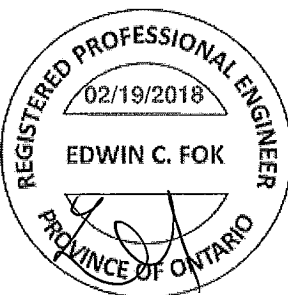
PLATE PLACEMENT TOL. = 0.250 inches

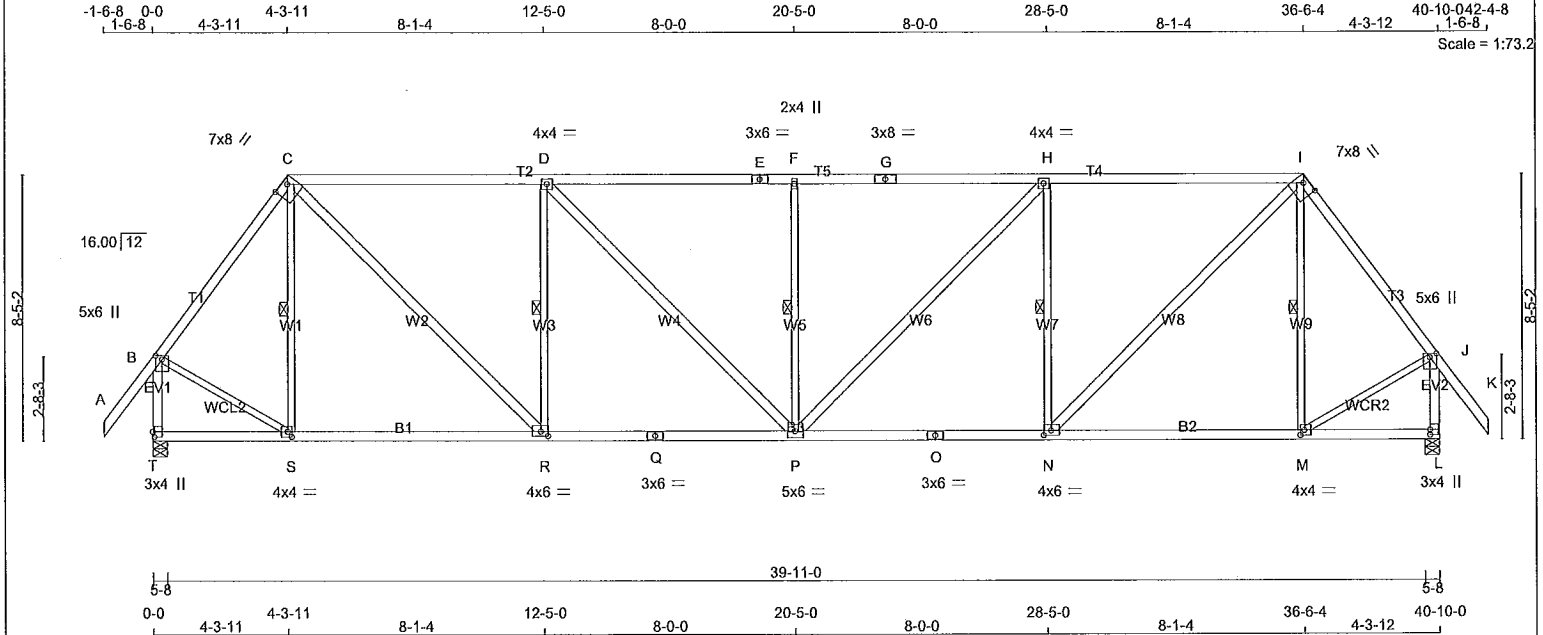
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (R) (INPUT = 0.90)
JSI METAL= 0.88 (O) (INPUT = 1.00)

A-18023956

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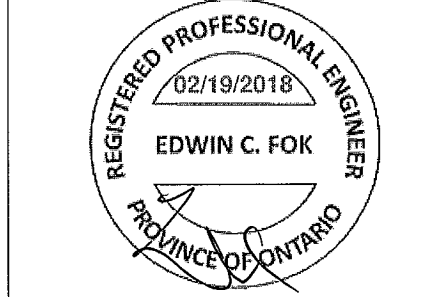


LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4 DRY	No.2	SPF
C - E	2x4 DRY	1650F 1.5E	SPF
E - G	2x4 DRY	1650F 1.5E	SPF
G - I	2x4 DRY	1650F 1.5E	SPF
I - K	2x4 DRY	No.2	SPF
K - L	2x4 DRY	No.2	SPF
L - J	2x4 DRY	No.2	SPF
J - Q	2x4 DRY	No.2	SPF
Q - O	2x4 DRY	No.2	SPF
O - L	2x4 DRY	No.2	SPF
ALL WEBS 2x3 DRY No.2 SPF			
EXCEPT			
C - R	2x4 DRY	No.2	SPF
D - P	2x4 DRY	No.2	SPF
P - H	2x4 DRY	No.2	SPF
N - I	2x4 DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	5.0	8.0	1.50 2.50
C	TTVW-h	MT20	7.0	8.0	1.75 5.00
D	TMVW-t	MT20	4.0	4.0	
E	TS-t	MT20	3.0	6.0	
F	TMVW-w	MT20	2.0	4.0	
G	TS-t	MT20	3.0	8.0	
H	TMVW-t	MT20	4.0	4.0	
I	TTVW-h	MT20	7.0	8.0	1.75 5.00
J	TMVW+p	MT20	5.0	6.0	1.50 2.50
L	BMV1+p	MT20	3.0	4.0	2.00
M	BMVW-t	MT20	4.0	4.0	2.00 1.50
N	BMVW-t	MT20	4.0	6.0	1.75 2.75
O	BS-t	MT20	3.0	6.0	
P	BMVWW-t	MT20	5.0	6.0	
Q	BS-t	MT20	3.0	6.0	
R	BMVW-t	MT20	4.0	6.0	1.75 2.75
S	BMVW-t	MT20	4.0	4.0	2.00 1.50
T	BMV1+p	MT20	3.0	4.0	2.00 0.50

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQRD
		GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	SNOW	DOWN	HORZ	UPLIFT
T	2220	0	2220	0	5-8
L	2220	0	2220	0	5-8

UNFACTORED REACTIONS		1ST LCASE	MAX/MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
T	1590	927 / 0	0 / 0	0 / 0	0 / 0	664 / 0	0 / 0		
L	1590	927 / 0	0 / 0	0 / 0	0 / 0	664 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING
FOR SECTION C-I, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-S, D-R, F-P, H-N, I-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	
FR-TO		FROM TO		FR-TO			
A-B	0 / 52	-78.0 -78.0	0.15 (1)	10.00	S-C	-399 / 0	0.18 (1)
B-C	-1707 / 0	-78.0 -78.0	0.26 (1)	4.89	C-R	0 / 1860	0.30 (1)
C-D	-2330 / 0	-85.5 -85.5	0.66 (1)	2.00	R-D	-1161 / 0	0.52 (1)
D-E	-2711 / 0	-85.5 -85.5	0.64 (1)	2.00	D-P	0 / 544	0.09 (1)
E-F	-2711 / 0	-85.5 -85.5	0.64 (1)	2.00	P-F	-631 / 0	0.29 (1)
F-G	-2711 / 0	-85.5 -85.5	0.64 (1)	2.00	P-H	0 / 544	0.09 (1)
G-H	-2711 / 0	-85.5 -85.5	0.64 (1)	2.00	H-H	-1161 / 0	0.52 (1)
H-I	-2330 / 0	-85.5 -85.5	0.66 (1)	2.00	N-I	0 / 1860	0.30 (1)
I-J	-1707 / 0	-78.0 -78.0	0.26 (1)	4.89	M-I	-399 / 0	0.18 (1)
J-K	0 / 52	-78.0 -78.0	0.15 (1)	10.00	B-S	0 / 1148	0.26 (1)
T-B	-2197 / 0	0.0 0.0	0.32 (1)	5.74	M-J	0 / 1148	0.26 (1)
L-J	-2197 / 0	0.0 0.0	0.32 (1)	5.74			
T-S	0 / 0	-18.5 -18.5	0.18 (4)	10.00			
S-R	0 / 1014	-18.5 -18.5	0.35 (4)	10.00			
R-Q	0 / 2330	-18.5 -18.5	0.51 (1)	10.00			
Q-P	0 / 2330	-18.5 -18.5	0.51 (1)	10.00			
P-O	0 / 2330	-18.5 -18.5	0.51 (1)	10.00			
O-N	0 / 2330	-18.5 -18.5	0.51 (1)	10.00			
N-M	0 / 1014	-18.5 -18.5	0.35 (4)	10.00			
M-L	0 / 0	-18.5 -18.5	0.18 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.36")
CALCULATED VERT. DEFL.(LL) = L/999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (1.36")
CALCULATED VERT. DEFL.(TL) = L/999 (0.28")

CSI: TC=0.66/1.00 (H-I:1), BC=0.51/1.00 (N-P:1), WB=0.52/1.00 (D-R:1), SI=0.33/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

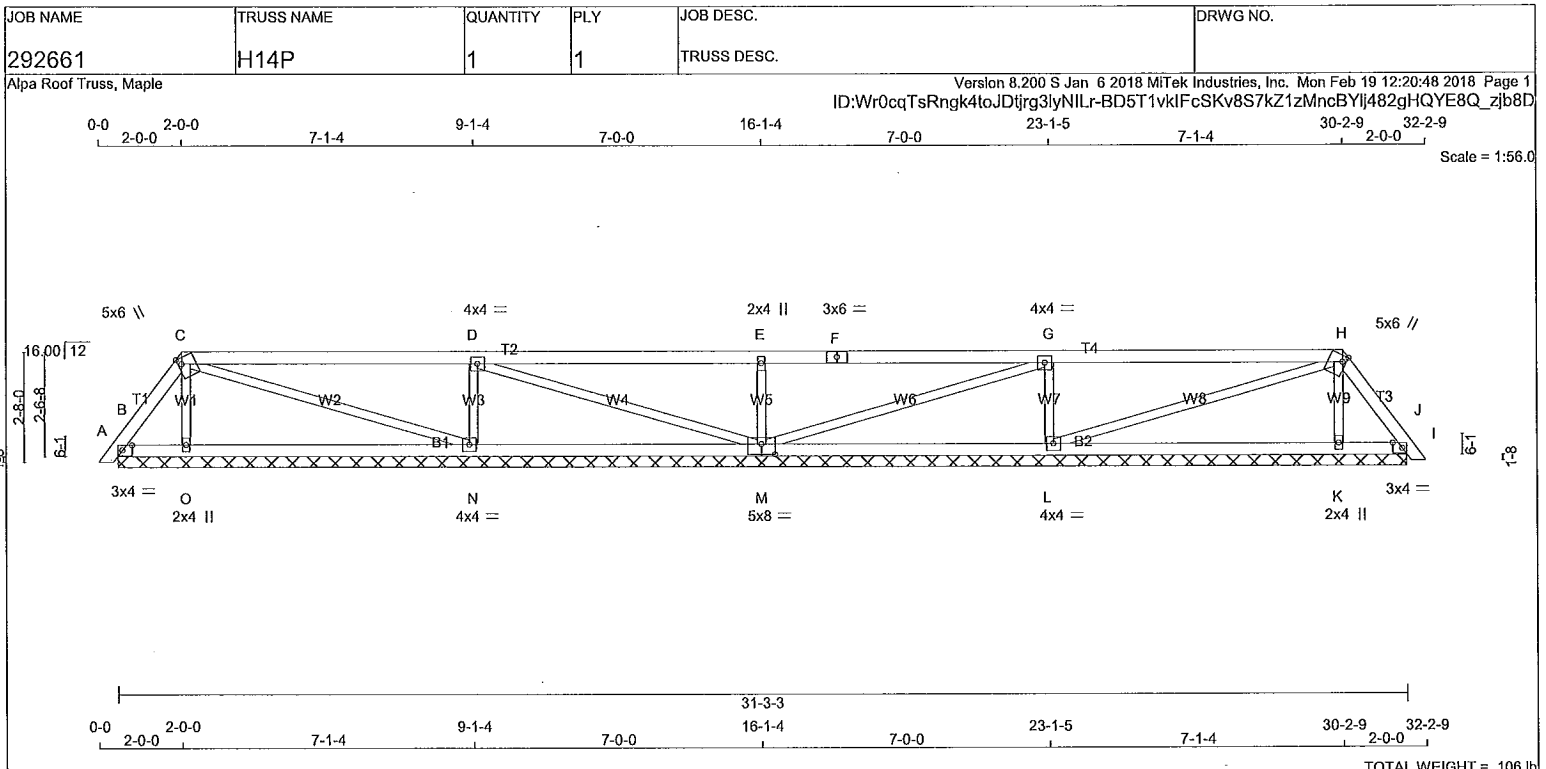
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (B) (INPUT = 0.90)
JSI METAL= 0.80 (C) (INPUT = 1.00)



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4 DRY	No.2	SPF
C - F	2x4 DRY	No.2	SPF
F - H	2x4 DRY	No.2	SPF
H - J	2x4 DRY	No.2	SPF
B - M	2x4 DRY	No.2	SPF
M - I	2x4 DRY	No.2	SPF
ALL WEBS	2x3 DRY	No.2	SPF
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0	1.50 2.75
C	TTWW+m	MT20	5.0	6.0	1.75 1.00
D	TMWW-I	MT20	4.0	4.0	
E	TMWW+w	MT20	2.0	4.0	
F	TS-I	MT20	3.0	6.0	
G	TMWW-I	MT20	4.0	4.0	
H	TTWW+m	MT20	5.0	6.0	1.75 1.00
I	TMB1-I	MT20	3.0	4.0	1.50 2.75
K	BMW1+w	MT20	2.0	4.0	
L	BMWW1-I	MT20	4.0	4.0	
M	BSWW1-I	MT20	5.0	8.0	3.00 4.00
N	BMWW1-I	MT20	4.0	4.0	
O	BMW1+w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
B	142	0	142	0	0	31-3-3 (15-7-9)9-7			
O	303	0	303	0	0	31-3-3 (15-7-9)9-7			
N	775	0	775	0	0	31-3-3 (15-7-9)9-7			
M	630	0	630	0	0	31-3-3 (15-7-9)9-7			
L	775	0	775	0	0	31-3-3 (15-7-9)9-7			
K	303	0	303	0	0	31-3-3 (15-7-9)9-7			
I	142	0	142	0	0	31-3-3 (15-7-9)9-7			

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN. COMPONENT REACTIONS		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	WIND	SOIL
B	97	86 / 0	0 / 0	0 / 0	0 / 0	11 / 0	0 / 0	0 / 0	0 / 0
O	221	107 / 0	0 / 0	0 / 0	0 / 0	115 / 0	0 / 0	0 / 0	0 / 0
N	552	336 / 0	0 / 0	0 / 0	0 / 0	217 / 0	0 / 0	0 / 0	0 / 0
M	447	285 / 0	0 / 0	0 / 0	0 / 0	162 / 0	0 / 0	0 / 0	0 / 0
L	552	336 / 0	0 / 0	0 / 0	0 / 0	217 / 0	0 / 0	0 / 0	0 / 0
K	221	107 / 0	0 / 0	0 / 0	0 / 0	115 / 0	0 / 0	0 / 0	0 / 0
I	97	86 / 0	0 / 0	0 / 0	0 / 0	11 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, O, N, M, L, K, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	MAX. UNBRACED LENGTH	MEMB.	FORCE (LBS)	MAX. FACTORED (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 10	-78.0 -78.0	0.01 (1)	10.00	O-C	-199 / 0	0.03 (1)
B-C	-88 / 0	-78.0 -78.0	0.03 (1)	6.25	C-N	-24 / 0	0.03 (1)
C-D	-17 / 0	-78.0 -78.0	0.57 (1)	6.25	N-D	-617 / 0	0.10 (1)
D-E	0 / 23	-78.0 -78.0	0.57 (1)	10.00	D-M	-43 / 0	0.05 (1)
E-F	0 / 23	-78.0 -78.0	0.57 (1)	10.00	M-E	-503 / 0	0.08 (1)
F-G	0 / 23	-78.0 -78.0	0.57 (1)	10.00	M-G	-43 / 0	0.05 (1)
G-H	-17 / 0	-78.0 -78.0	0.57 (1)	6.25	L-G	-617 / 0	0.10 (1)
H-I	-88 / 0	-78.0 -78.0	0.03 (1)	6.25	L-H	-24 / 0	0.03 (1)
I-J	0 / 10	-78.0 -78.0	0.01 (1)	10.00	K-H	-199 / 0	0.03 (1)
B-O	0 / 50	-18.5 -18.5	0.13 (4)	10.00			
O-N	0 / 41	-18.5 -18.5	0.24 (4)	10.00			
N-M	0 / 18	-18.5 -18.5	0.24 (4)	10.00			
M-L	0 / 18	-18.5 -18.5	0.24 (4)	10.00			
L-K	0 / 41	-18.5 -18.5	0.24 (4)	10.00			
K-I	0 / 50	-18.5 -18.5	0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI=0.57/1.00 (E-G:1), BC=0.24/1.00 (N-O:4), WB=0.10/1.00 (D-N:1), SSI=0.26/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

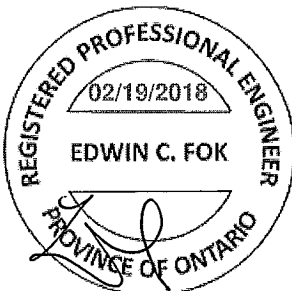
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

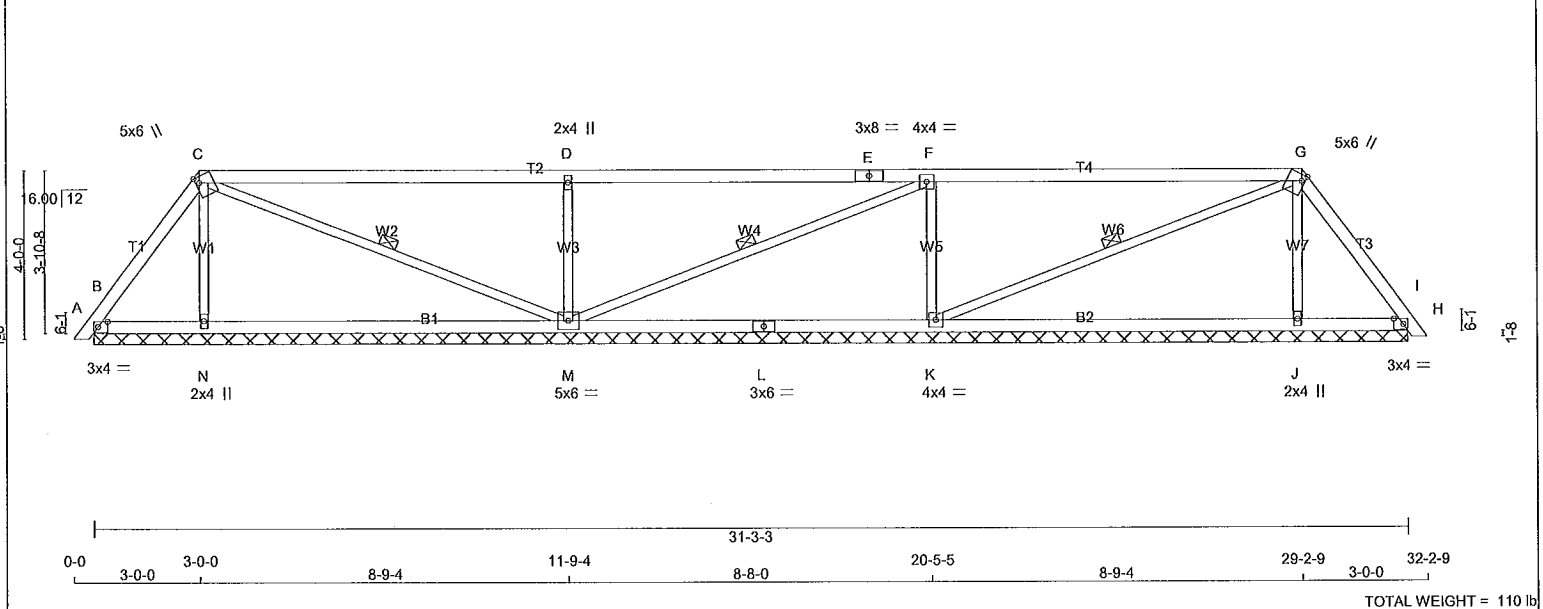
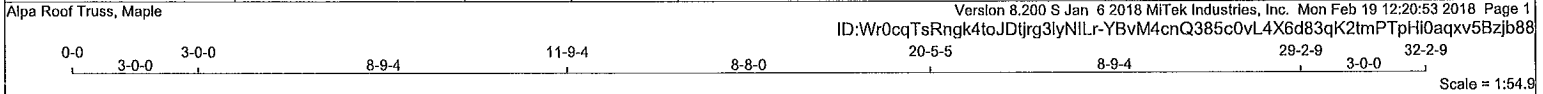
JSI GRIP= 0.75 (L) (INPUT = 0.90)
JSI METAL= 0.27 (M) (INPUT = 1.00)

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



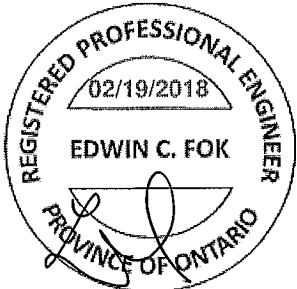
A-18023958

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292661	H15P	1	1	TRUSS DESC.	

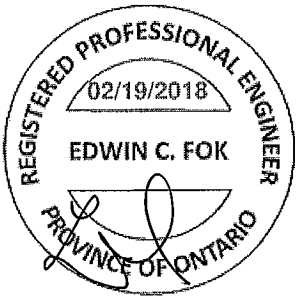


LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. A - C 2x4 DRY No.2 SPF C - E 2x4 DRY 2100F 1.8E SPF E - G 2x4 DRY 2100F 1.8E SPF G - I 2x4 DRY No.2 SPF B - L 2x4 DRY No.2 SPF L - H 2x4 DRY No.2 SPF ALL WEBS 2x3 DRY No.2 SPF DRY: SEASONED LUMBER.				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS <table><tr><th rowspan="2">JT</th><th colspan="2">FACTORED GROSS REACTION</th><th colspan="2">MAXIMUM FACTORED GROSS REACTION</th><th rowspan="2">INPUT BRG</th><th rowspan="2">REQRD BRG</th></tr><tr><th>VERT</th><th>HORZ</th><th>DOWN</th><th>HORZ</th></tr><tr><td>B</td><td>197</td><td>0</td><td>197</td><td>0</td><td>31-3-3 (15-3-214-15</td><td></td></tr><tr><td>N</td><td>401</td><td>0</td><td>401</td><td>0</td><td>31-3-3 (15-3-214-15</td><td></td></tr><tr><td>M</td><td>960</td><td>0</td><td>960</td><td>0</td><td>31-3-3 (15-3-214-15</td><td></td></tr><tr><td>K</td><td>900</td><td>0</td><td>900</td><td>0</td><td>31-3-3 (15-3-214-15</td><td></td></tr><tr><td>J</td><td>389</td><td>0</td><td>389</td><td>0</td><td>31-3-3 (15-3-214-15</td><td></td></tr><tr><td>H</td><td>223</td><td>0</td><td>223</td><td>0</td><td>31-3-3 (15-3-214-15</td><td></td></tr></table> VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH UNFACTORED REACTIONS <table><tr><th rowspan="2">JT</th><th colspan="2">1ST LCASE COMBINED</th><th colspan="2">MAX/MIN. COMPONENT REACTIONS</th><th rowspan="2">WIND</th><th rowspan="2">DEAD</th><th rowspan="2">SOIL</th></tr><tr><th>SNOW</th><th>LIVE</th><th>PERM.LIVE</th><th></th></tr><tr><td>B</td><td>135</td><td>112 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>24 / 0</td><td>0 / 0</td></tr><tr><td>N</td><td>292</td><td>143 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>149 / 0</td><td>0 / 0</td></tr><tr><td>M</td><td>682</td><td>429 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>254 / 0</td><td>0 / 0</td></tr><tr><td>K</td><td>641</td><td>397 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>244 / 0</td><td>0 / 0</td></tr><tr><td>J</td><td>284</td><td>137 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>147 / 0</td><td>0 / 0</td></tr><tr><td>H</td><td>154</td><td>126 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>28 / 0</td><td>0 / 0</td></tr></table> BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, N, M, K, J, H BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. 1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-M, F-M, G-K. LOADING TOTAL LOAD CASES: (4) <table><tr><th rowspan="2">MEMB.</th><th colspan="2">CHORDS MAX. FACTORED FORCE (LBS)</th><th colspan="2">FACTORED VERT. LOAD LC1 (PLF)</th><th rowspan="2">MAX. CS1 (LC)</th><th rowspan="2">MAX. UNBRAC LENGTH</th><th rowspan="2">MEMB. FR-TO</th><th colspan="2">WEBS MAX. FACTORED FORCE (LBS)</th><th rowspan="2">MAX. CS1 (LC)</th></tr><tr><th>FROM</th><th>TO</th><th>FROM</th><th>TO</th><th>FROM</th><th>TO</th></tr><tr><td>A-B</td><td>0 / 10</td><td></td><td>-78.0</td><td>-78.0</td><td>0.01 (1)</td><td>10.00</td><td>N-C</td><td>-266 / 0</td><td></td><td>0.06 (1)</td></tr><tr><td>B-C</td><td>-100 / 0</td><td></td><td>-78.0</td><td>-78.0</td><td>0.09 (1)</td><td>6.25</td><td>C-M</td><td>-69 / 0</td><td></td><td>0.04 (1)</td></tr><tr><td>C-D</td><td>0 / 14</td><td></td><td>-78.0</td><td>-78.0</td><td>0.53 (1)</td><td>10.00</td><td>M-D</td><td>-748 / 0</td><td></td><td>0.18 (1)</td></tr><tr><td>D-E</td><td>0 / 14</td><td></td><td>-78.0</td><td>-78.0</td><td>0.53 (1)</td><td>10.00</td><td>M-F</td><td>-59 / 0</td><td></td><td>0.04 (1)</td></tr><tr><td>E-F</td><td>0 / 14</td><td></td><td>-78.0</td><td>-78.0</td><td>0.53 (1)</td><td>10.00</td><td>K-F</td><td>-725 / 0</td><td></td><td>0.17 (1)</td></tr><tr><td>F-G</td><td>-40 / 0</td><td></td><td>-78.0</td><td>-78.0</td><td>0.53 (1)</td><td>6.25</td><td>K-G</td><td>-31 / 0</td><td></td><td>0.02 (1)</td></tr><tr><td>G-H</td><td>-132 / 0</td><td></td><td>-78.0</td><td>-78.0</td><td>0.09 (1)</td><td>6.25</td><td>J-G</td><td>-254 / 0</td><td></td><td>0.06 (1)</td></tr><tr><td>H-I</td><td>0 / 10</td><td></td><td>-78.0</td><td>-78.0</td><td>0.01 (1)</td><td>10.00</td><td></td><td></td><td></td><td></td></tr><tr><td>B-N</td><td>0 / 58</td><td></td><td>-18.5</td><td>-18.5</td><td>0.22 (4)</td><td>10.00</td><td></td><td></td><td></td><td></td></tr><tr><td>N-M</td><td>0 / 50</td><td></td><td>-18.5</td><td>-18.5</td><td>0.29 (4)</td><td>10.00</td><td></td><td></td><td></td><td></td></tr><tr><td>M-L</td><td>0 / 41</td><td></td><td>-18.5</td><td>-18.5</td><td>0.29 (4)</td><td>10.00</td><td></td><td></td><td></td><td></td></tr><tr><td>L-K</td><td>0 / 41</td><td></td><td>-18.5</td><td>-18.5</td><td>0.29 (4)</td><td>10.00</td><td></td><td></td><td></td><td></td></tr><tr><td>K-J</td><td>0 / 69</td><td></td><td>-18.5</td><td>-18.5</td><td>0.29 (4)</td><td>10.00</td><td></td><td></td><td></td><td></td></tr><tr><td>J-H</td><td>0 / 77</td><td></td><td>-18.5</td><td>-18.5</td><td>0.22 (4)</td><td>10.00</td><td></td><td></td><td></td><td></td></tr></table>				JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG	VERT	HORZ	DOWN	HORZ	B	197	0	197	0	31-3-3 (15-3-214-15		N	401	0	401	0	31-3-3 (15-3-214-15		M	960	0	960	0	31-3-3 (15-3-214-15		K	900	0	900	0	31-3-3 (15-3-214-15		J	389	0	389	0	31-3-3 (15-3-214-15		H	223	0	223	0	31-3-3 (15-3-214-15		JT	1ST LCASE COMBINED		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL	SNOW	LIVE	PERM.LIVE		B	135	112 / 0	0 / 0	0 / 0	0 / 0	24 / 0	0 / 0	N	292	143 / 0	0 / 0	0 / 0	0 / 0	149 / 0	0 / 0	M	682	429 / 0	0 / 0	0 / 0	0 / 0	254 / 0	0 / 0	K	641	397 / 0	0 / 0	0 / 0	0 / 0	244 / 0	0 / 0	J	284	137 / 0	0 / 0	0 / 0	0 / 0	147 / 0	0 / 0	H	154	126 / 0	0 / 0	0 / 0	0 / 0	28 / 0	0 / 0	MEMB.	CHORDS MAX. FACTORED FORCE (LBS)		FACTORED VERT. LOAD LC1 (PLF)		MAX. CS1 (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	WEBS MAX. FACTORED FORCE (LBS)		MAX. CS1 (LC)	FROM	TO	FROM	TO	FROM	TO	A-B	0 / 10		-78.0	-78.0	0.01 (1)	10.00	N-C	-266 / 0		0.06 (1)	B-C	-100 / 0		-78.0	-78.0	0.09 (1)	6.25	C-M	-69 / 0		0.04 (1)	C-D	0 / 14		-78.0	-78.0	0.53 (1)	10.00	M-D	-748 / 0		0.18 (1)	D-E	0 / 14		-78.0	-78.0	0.53 (1)	10.00	M-F	-59 / 0		0.04 (1)	E-F	0 / 14		-78.0	-78.0	0.53 (1)	10.00	K-F	-725 / 0		0.17 (1)	F-G	-40 / 0		-78.0	-78.0	0.53 (1)	6.25	K-G	-31 / 0		0.02 (1)	G-H	-132 / 0		-78.0	-78.0	0.09 (1)	6.25	J-G	-254 / 0		0.06 (1)	H-I	0 / 10		-78.0	-78.0	0.01 (1)	10.00					B-N	0 / 58		-18.5	-18.5	0.22 (4)	10.00					N-M	0 / 50		-18.5	-18.5	0.29 (4)	10.00					M-L	0 / 41		-18.5	-18.5	0.29 (4)	10.00					L-K	0 / 41		-18.5	-18.5	0.29 (4)	10.00					K-J	0 / 69		-18.5	-18.5	0.29 (4)	10.00					J-H	0 / 77		-18.5	-18.5	0.22 (4)	10.00				
JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG																																																																																																																																																																																																																																																																																													
	VERT	HORZ	DOWN	HORZ																																																																																																																																																																																																																																																																																															
B	197	0	197	0	31-3-3 (15-3-214-15																																																																																																																																																																																																																																																																																														
N	401	0	401	0	31-3-3 (15-3-214-15																																																																																																																																																																																																																																																																																														
M	960	0	960	0	31-3-3 (15-3-214-15																																																																																																																																																																																																																																																																																														
K	900	0	900	0	31-3-3 (15-3-214-15																																																																																																																																																																																																																																																																																														
J	389	0	389	0	31-3-3 (15-3-214-15																																																																																																																																																																																																																																																																																														
H	223	0	223	0	31-3-3 (15-3-214-15																																																																																																																																																																																																																																																																																														
JT	1ST LCASE COMBINED		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL																																																																																																																																																																																																																																																																																												
	SNOW	LIVE	PERM.LIVE																																																																																																																																																																																																																																																																																																
B	135	112 / 0	0 / 0	0 / 0	0 / 0	24 / 0	0 / 0																																																																																																																																																																																																																																																																																												
N	292	143 / 0	0 / 0	0 / 0	0 / 0	149 / 0	0 / 0																																																																																																																																																																																																																																																																																												
M	682	429 / 0	0 / 0	0 / 0	0 / 0	254 / 0	0 / 0																																																																																																																																																																																																																																																																																												
K	641	397 / 0	0 / 0	0 / 0	0 / 0	244 / 0	0 / 0																																																																																																																																																																																																																																																																																												
J	284	137 / 0	0 / 0	0 / 0	0 / 0	147 / 0	0 / 0																																																																																																																																																																																																																																																																																												
H	154	126 / 0	0 / 0	0 / 0	0 / 0	28 / 0	0 / 0																																																																																																																																																																																																																																																																																												
MEMB.	CHORDS MAX. FACTORED FORCE (LBS)		FACTORED VERT. LOAD LC1 (PLF)		MAX. CS1 (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	WEBS MAX. FACTORED FORCE (LBS)		MAX. CS1 (LC)																																																																																																																																																																																																																																																																																									
	FROM	TO	FROM	TO				FROM	TO																																																																																																																																																																																																																																																																																										
A-B	0 / 10		-78.0	-78.0	0.01 (1)	10.00	N-C	-266 / 0		0.06 (1)																																																																																																																																																																																																																																																																																									
B-C	-100 / 0		-78.0	-78.0	0.09 (1)	6.25	C-M	-69 / 0		0.04 (1)																																																																																																																																																																																																																																																																																									
C-D	0 / 14		-78.0	-78.0	0.53 (1)	10.00	M-D	-748 / 0		0.18 (1)																																																																																																																																																																																																																																																																																									
D-E	0 / 14		-78.0	-78.0	0.53 (1)	10.00	M-F	-59 / 0		0.04 (1)																																																																																																																																																																																																																																																																																									
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F-G	-40 / 0		-78.0	-78.0	0.53 (1)	6.25	K-G	-31 / 0		0.02 (1)																																																																																																																																																																																																																																																																																									
G-H	-132 / 0		-78.0	-78.0	0.09 (1)	6.25	J-G	-254 / 0		0.06 (1)																																																																																																																																																																																																																																																																																									
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M-L	0 / 41		-18.5	-18.5	0.29 (4)	10.00																																																																																																																																																																																																																																																																																													
L-K	0 / 41		-18.5	-18.5	0.29 (4)	10.00																																																																																																																																																																																																																																																																																													
K-J	0 / 69		-18.5	-18.5	0.29 (4)	10.00																																																																																																																																																																																																																																																																																													
J-H	0 / 77		-18.5	-18.5	0.22 (4)	10.00																																																																																																																																																																																																																																																																																													
PLATES (table is in inches) <table><tr><th>JT</th><th>TYPE</th><th>PLATES</th><th>W</th><th>LEN</th><th>Y</th><th>X</th></tr><tr><td>B</td><td>TMB1-I</td><td>MT20</td><td>3.0</td><td>4.0</td><td>1.50</td><td>2.75</td></tr><tr><td>C</td><td>TTWW+m</td><td>MT20</td><td>5.0</td><td>6.0</td><td>1.75</td><td>1.00</td></tr><tr><td>D</td><td>TMW+w</td><td>MT20</td><td>2.0</td><td>4.0</td><td></td><td></td></tr><tr><td>E</td><td>TS-I</td><td>MT20</td><td>3.0</td><td>8.0</td><td></td><td></td></tr><tr><td>F</td><td>TMWW-I</td><td>MT20</td><td>4.0</td><td>4.0</td><td></td><td></td></tr><tr><td>G</td><td>TTWW+m</td><td>MT20</td><td>5.0</td><td>6.0</td><td>1.75</td><td>1.00</td></tr><tr><td>H</td><td>TMB1-I</td><td>MT20</td><td>3.0</td><td>4.0</td><td>1.50</td><td>2.75</td></tr><tr><td>J</td><td>BMW1+w</td><td>MT20</td><td>2.0</td><td>4.0</td><td></td><td></td></tr><tr><td>K</td><td>BMWW1-I</td><td>MT20</td><td>4.0</td><td>4.0</td><td></td><td></td></tr><tr><td>L</td><td>BS-I</td><td>MT20</td><td>3.0</td><td>6.0</td><td></td><td></td></tr><tr><td>M</td><td>BMWWW1-I</td><td>MT20</td><td>5.0</td><td>6.0</td><td></td><td></td></tr><tr><td>N</td><td>BMW1+w</td><td>MT20</td><td>2.0</td><td>4.0</td><td></td><td></td></tr></table>				JT	TYPE	PLATES	W	LEN	Y	X	B	TMB1-I	MT20	3.0	4.0	1.50	2.75	C	TTWW+m	MT20	5.0	6.0	1.75	1.00	D	TMW+w	MT20	2.0	4.0			E	TS-I	MT20	3.0	8.0			F	TMWW-I	MT20	4.0	4.0			G	TTWW+m	MT20	5.0	6.0	1.75	1.00	H	TMB1-I	MT20	3.0	4.0	1.50	2.75	J	BMW1+w	MT20	2.0	4.0			K	BMWW1-I	MT20	4.0	4.0			L	BS-I	MT20	3.0	6.0			M	BMWWW1-I	MT20	5.0	6.0			N	BMW1+w	MT20	2.0	4.0			DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 21.0 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.4 PSF TOTAL LOAD = 34.4 PSF SPACING = 24.0 IN.C/C LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012, OBC 2018 - CSA 086-09, CSA 086-14 - TPIC 2011, TPIC 2014 (55% OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD CSI: TC=0.53/1.00 (F-G:1) , BC=0.29/1.00 (J-K:4) , WB=0.18/1.00 (D-M:1) , SSI=0.32/1.00 (F-G:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10 COMPANION LIVE LOAD FACTOR = 1.00 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT . NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 618 354 1667 788 1987 1656 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.84 (M) (INPUT = 0.90) JSI METAL= 0.48 (E) (INPUT = 1.00)																																																																																																																																																																																																				
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NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



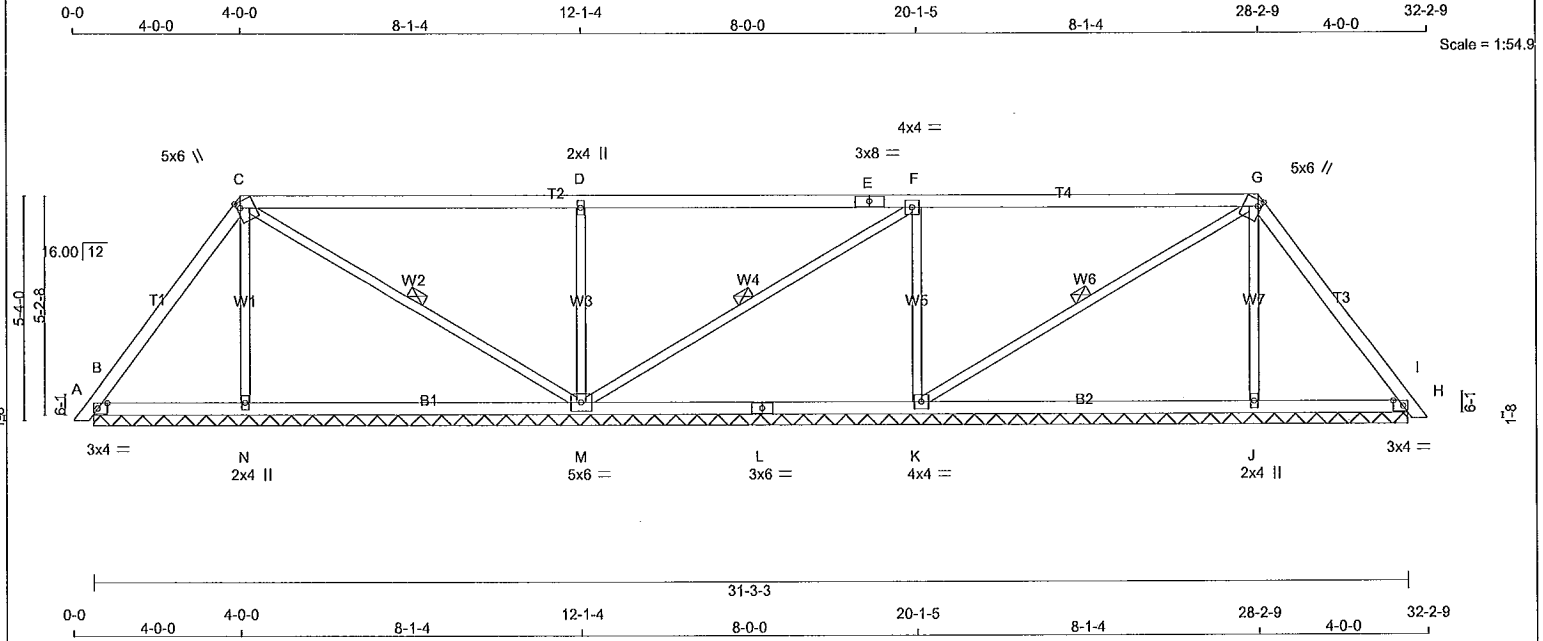
A-18023959



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292661	H16P	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Feb 19 12:20:58 2018 Page 1
ID:Wr0cqTsRngk4toJDjrg3lyNlR-u8lF8KrZuglv6gD2KfDJmu1t6n7HUVWMIj6gnPzjb83



TOTAL WEIGHT = 2 X 116 = 231 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0	1.50 2.75
C	TTWW+m	MT20	5.0	6.0	1.75 1.00
D	TMW+w	MT20	2.0	4.0	
E	TS-t	MT20	3.0	8.0	
F	TMWW-t	MT20	4.0	4.0	
G	TTWW+m	MT20	5.0	6.0	1.75 1.00
H	TMB1-I	MT20	3.0	4.0	1.50 2.75
J	BMW1+w	MT20	2.0	4.0	
K	BMWW1-t	MT20	4.0	4.0	
L	BS-t	MT20	3.0	6.0	
M	BMWWW1-t	MT20	5.0	6.0	
N	BMW1+w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
B	262	0	262	0	31-3-3 (15-3-214-6	
N	377	0	377	0	31-3-3 (15-3-214-6	
M	948	0	948	0	31-3-3 (15-3-214-6	
K	811	0	811	0	31-3-3 (15-3-214-6	
J	368	0	368	0	31-3-3 (15-3-214-6	
H	304	0	304	0	31-3-3 (15-3-214-6	

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS							
JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL	
		SNOW	LIVE				
B	183	134 / 0	0 / 0	0 / 0	49 / 0	0 / 0	
N	275	136 / 0	0 / 0	0 / 0	139 / 0	0 / 0	
M	673	429 / 0	0 / 0	0 / 0	244 / 0	0 / 0	
K	578	355 / 0	0 / 0	0 / 0	223 / 0	0 / 0	
J	268	131 / 0	0 / 0	0 / 0	137 / 0	0 / 0	
H	212	157 / 0	0 / 0	0 / 0	55 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, N, M, K, J, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-M, F-M, G-K.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 10	-78.0 -78.0	0.01 (1)	N-C	-252 / 0	0.10 (1)	
B-C	-105 / 0	-78.0 -78.0	0.17 (1)	C-M	-105 / 0	0.06 (1)	
C-D	0 / 34	-78.0 -78.0	0.69 (1)	M-D	-691 / 0	0.28 (1)	
D-E	0 / 34	-78.0 -78.0	0.69 (1)	M-F	-100 / 0	0.06 (1)	
E-F	0 / 34	-78.0 -78.0	0.69 (1)	K-F	-638 / 0	0.26 (1)	
F-G	-51 / 0	-78.0 -78.0	0.69 (1)	K-G	-41 / 0	0.03 (1)	
G-H	-157 / 0	-78.0 -78.0	0.17 (1)	J-G	-243 / 0	0.10 (1)	
H-I	0 / 10	-78.0 -78.0	0.01 (1)				
B-N	0 / 61	-18.5 -18.5	0.18 (4)				
N-M	0 / 56	-18.5 -18.5	0.25 (4)				
M-L	0 / 51	-18.5 -18.5	0.24 (4)				
L-K	0 / 51	-18.5 -18.5	0.24 (4)				
K-J	0 / 87	-18.5 -18.5	0.25 (4)				
J-H	0 / 92	-18.5 -18.5	0.18 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.69/1.00 (D-F:1), BC=0.25/1.00 (J-K:4), WB=0.28/1.00 (D-M:1), SSI=0.30/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667 788 1987 1656

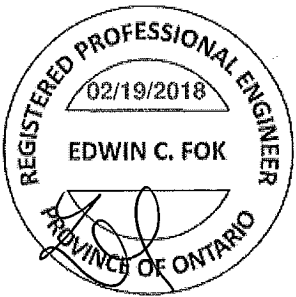
PLATE PLACEMENT TOL. = 0.250 inches

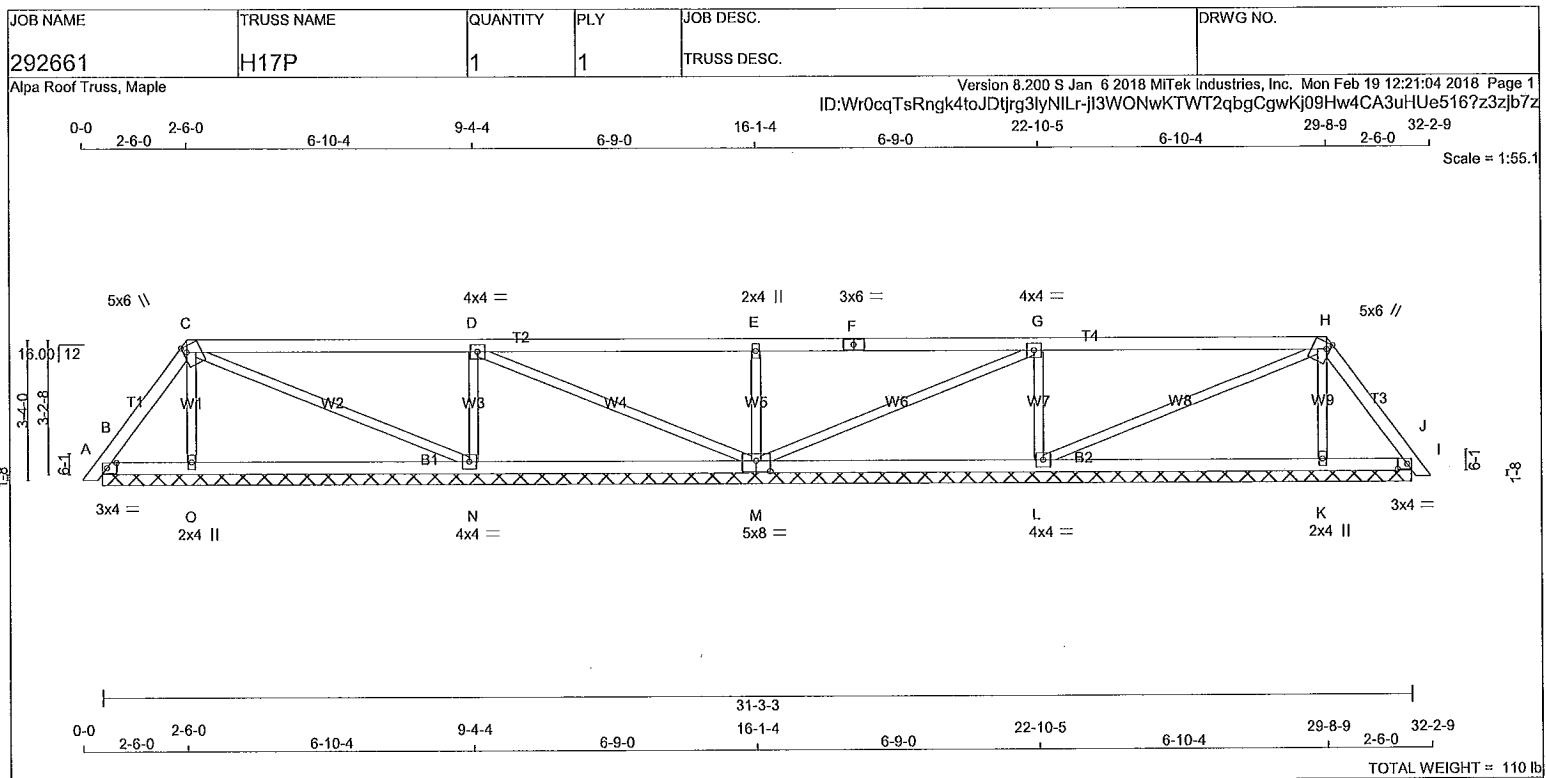
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.73 (E) (INPUT = 0.90)

JSI METAL= 0.57 (E) (INPUT = 1.00)

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design





LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
B - M	2x4	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0	1.50 2.75
C	TTWW+m	MT20	5.0	6.0	1.75 1.00
D	TMWW-I	MT20	4.0	4.0	
E	TMW+w	MT20	2.0	4.0	
F	TS-I	MT20	3.0	6.0	
G	TMWW-I	MT20	4.0	4.0	
H	TTWW+m	MT20	5.0	6.0	1.75 1.00
I	TMB1-I	MT20	3.0	4.0	1.50 2.75
K	BMW1+w	MT20	2.0	4.0	
L	BMWW1-I	MT20	4.0	4.0	
M	BSWW1-I	MT20	5.0	8.0	3.00 4.00
N	BMWW1-I	MT20	4.0	4.0	
O	BMW1+w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	HORZ	BRG	IN-SX	BRG	IN-SX
B	186	0	186	0	0	31-3-3	(15-7-9)0-6		
O	293	0	293	0	0	31-3-3	(15-7-9)0-6		
N	738	0	738	0	0	31-3-3	(15-7-9)0-6		
M	636	0	636	0	0	31-3-3	(15-7-9)0-6		
L	738	0	738	0	0	31-3-3	(15-7-9)0-6		
K	293	0	293	0	0	31-3-3	(15-7-9)0-6		
I	186	0	186	0	0	31-3-3	(15-7-9)0-6		

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	128	101/0	0/0	0/0	0/0	27/0	0/0
O	214	106/0	0/0	0/0	0/0	108/0	0/0
N	527	319/0	0/0	0/0	0/0	208/0	0/0
M	451	290/0	0/0	0/0	0/0	161/0	0/0
L	527	319/0	0/0	0/0	0/0	208/0	0/0
K	214	106/0	0/0	0/0	0/0	108/0	0/0
I	128	101/0	0/0	0/0	0/0	27/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, O, N, M, L, K, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED MAX CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 10	-78.0	-78.0 0.01 (1)	10.00	O-C	-196 / 0	0.04 (1)
B-C	-100 / 0	-78.0	-78.0 0.05 (1)	6.25	C-N	-31 / 0	0.03 (1)
C-D	-22 / 0	-78.0	-78.0 0.53 (1)	6.25	N-D	-581 / 0	0.11 (1)
D-E	0 / 40	-78.0	-78.0 0.54 (1)	10.00	D-M	-67 / 0	0.07 (1)
E-F	0 / 40	-78.0	-78.0 0.54 (1)	10.00	M-E	-485 / 0	0.09 (1)
F-G	0 / 40	-78.0	-78.0 0.54 (1)	10.00	M-G	-67 / 0	0.07 (1)
G-H	-22 / 0	-78.0	-78.0 0.53 (1)	6.25	L-G	-581 / 0	0.11 (1)
H-I	-100 / 0	-78.0	-78.0 0.05 (1)	6.25	L-H	-31 / 0	0.03 (1)
I-J	0 / 10	-78.0	-78.0 0.01 (1)	10.00	K-H	-196 / 0	0.04 (1)
B-O	0 / 58	-18.5	-18.5 0.11 (4)	10.00			
O-N	0 / 51	-18.5	-18.5 0.22 (4)	10.00			
N-M	0 / 22	-18.5	-18.5 0.22 (4)	10.00			
M-L	0 / 22	-18.5	-18.5 0.22 (4)	10.00			
L-K	0 / 51	-18.5	-18.5 0.22 (4)	10.00			
K-I	0 / 58	-18.5	-18.5 0.11 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 21.0	PSF
DL = 6.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.4	PSF	
TOTAL LOAD = 34.4	PSF	

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.54/1.00 (D-E:1), BC=0.22/1.00 (K-L:4), WB=0.11/1.00 (D-N:1), SSI=0.25/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

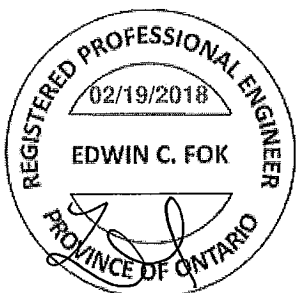
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1667 1656

PLATE PLACEMENT TOL. = 0.250 inches

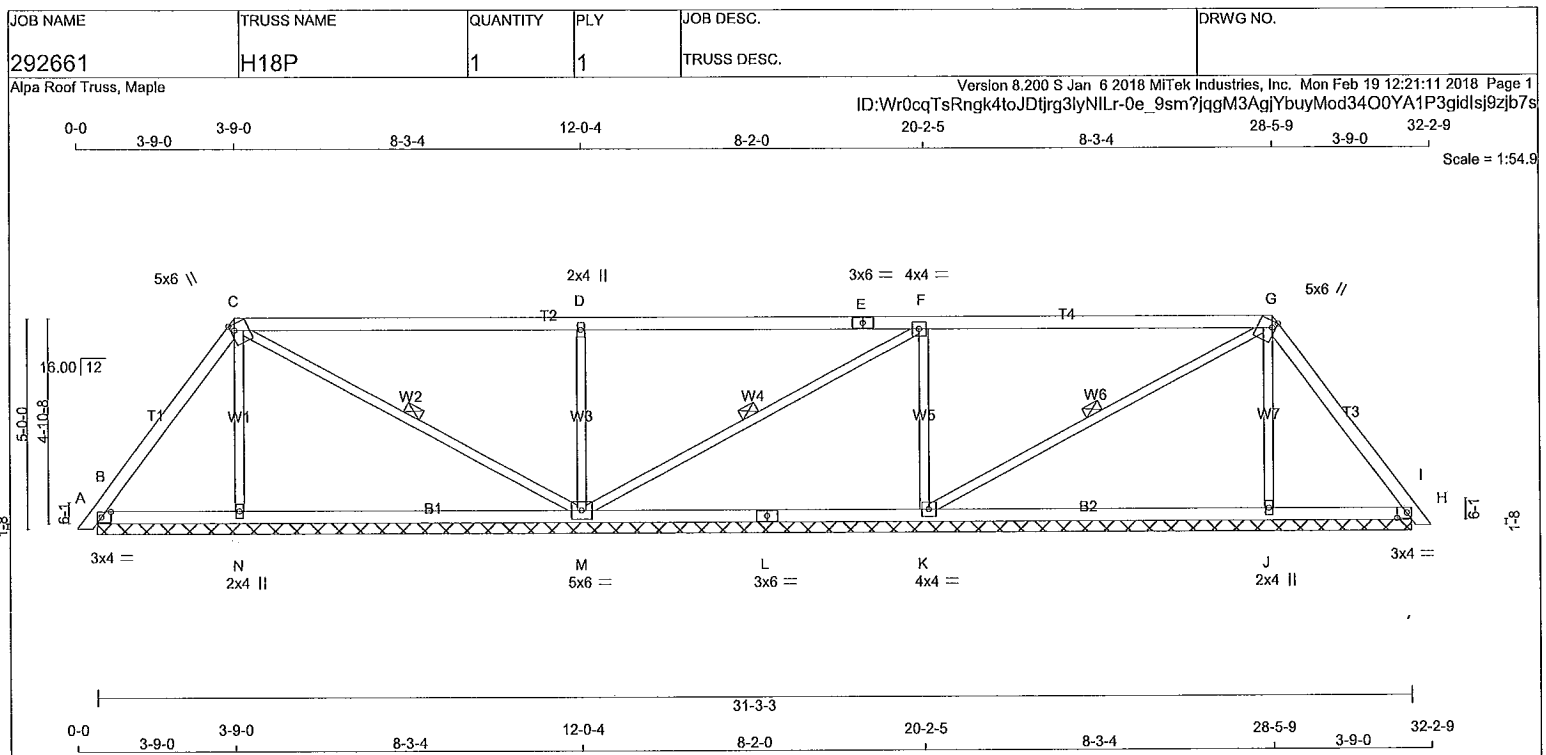
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.64 (C) (INPUT = 0.90)
JSI METAL= 0.29 (F) (INPUT = 1.00)

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



A-18023961



TOTAL WEIGHT = 114 lb

LUMBER				DESCR.	
N. L. G. A. RULES	SIZE	LUMBER			
CHORDS					
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
G - I	2x4	DRY	No.2	SPF	
B - L	2x4	DRY	No.2	SPF	
L - H	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0	1.50 2.75
C	TTWW+m	MT20	5.0	6.0	1.75 1.00
D	TMW+w	MT20	2.0	4.0	
E	TS-t	MT20	3.0	6.0	
F	TTWW-t	MT20	4.0	4.0	
G	TTWW+m	MT20	5.0	6.0	1.75 1.00
H	TMB1-I	MT20	3.0	4.0	1.50 2.75
J	BMW1+w	MT20	2.0	4.0	
K	BMWW1-I	MT20	4.0	4.0	
L	BS-t	MT20	3.0	6.0	
M	BMWW1-I	MT20	5.0	6.0	
N	BMW1+w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT		VERT	HORZ	DOWN	HORZ	IN-SX		IN-SX	
B	248	0	248	0	0	31-3-3 (15-3-214-2			
N	382	0	382	0	0	31-3-3 (15-3-214-2			
M	949	0	949	0	0	31-3-3 (15-3-214-2			
K	834	0	834	0	0	31-3-3 (15-3-214-2			
J	372	0	372	0	0	31-3-3 (15-3-214-2			
H	285	0	285	0	0	31-3-3 (15-3-214-2			

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	172	129 / 0	0 / 0	0 / 0	0 / 0	43 / 0	0 / 0
N	278	138 / 0	0 / 0	0 / 0	0 / 0	141 / 0	0 / 0
M	673	428 / 0	0 / 0	0 / 0	0 / 0	246 / 0	0 / 0
K	594	366 / 0	0 / 0	0 / 0	0 / 0	228 / 0	0 / 0
J	271	132 / 0	0 / 0	0 / 0	0 / 0	139 / 0	0 / 0
H	198	149 / 0	0 / 0	0 / 0	0 / 0	49 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, N, M, K, J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-M, F-M, G-K.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. (PLF)	LC1 MAX (LC)	MEMB.	FORCE (LBS)	MAX (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 10	-78.0	-78.0 0.01 (1)	10.00	N-C	-256 / 0	0.09 (1)
B-C	-104 / 0	-78.0	-78.0 0.14 (1)	6.25	C-M	-96 / 0	0.06 (1)
C-D	0 / 29	-78.0	-78.0 0.72 (1)	10.00	M-D	-705 / 0	0.25 (1)
D-E	0 / 28	-78.0	-78.0 0.72 (1)	10.00	M-F	-89 / 0	0.05 (1)
E-F	0 / 28	-78.0	-78.0 0.72 (1)	10.00	K-F	-661 / 0	0.23 (1)
F-G	-48 / 0	-78.0	-78.0 0.72 (1)	6.25	K-G	-39 / 0	0.02 (1)
G-H	-151 / 0	-78.0	-78.0 0.14 (1)	6.25	J-G	-246 / 0	0.09 (1)
H-I	0 / 10	-78.0	-78.0 0.01 (1)	10.00			
B-N	0 / 61	-18.5	-18.5 0.19 (4)	10.00			
N-M	0 / 55	-18.5	-18.5 0.26 (4)	10.00			
M-L	0 / 49	-18.5	-18.5 0.26 (4)	10.00			
L-K	0 / 49	-18.5	-18.5 0.26 (4)	10.00			
K-J	0 / 83	-18.5	-18.5 0.26 (4)	10.00			
J-H	0 / 88	-18.5	-18.5 0.19 (4)	10.00			

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.72/1.00 (D-F:1), BC=0.26/1.00 (J-K:4), WB=0.25/1.00 (D-M:1), SSI=0.30/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

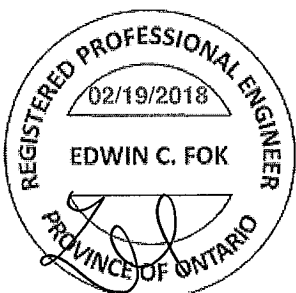
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (E) (INPUT = 0.90)
JSI METAL= 0.41 (E) (INPUT = 1.00)

A-18023962



Alpa Roof Truss, Maple

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ID:Wr0cqTsRngk4toJDlrg3lyNlR-9GNBS9b0AJL5u182p3OIisXF7mQXRfryughYEJzb_3

-1-6-8 0-0 1-9-12 8-1-0 14-3-0 20-5-0 26-7-0 32-9-0 39-0-4 40-10-042-4-8
1-6-8 1-9-12 6-3-4 6-2-0 6-2-0 6-2-0 6-2-0 6-3-4 1-9-12 1-6-8

Scale = 1:73.7

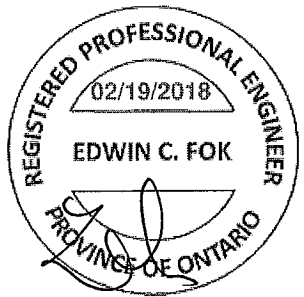


DRY, SEASONED LUMBER.

PLATES (table is in inches) 90% OF WEB LENGTH.

E-F	-11465 / 0	-78.0	-78.0	0.59 (1)	2.74	E-E	-1099 / 0	0.36 (1)
E-G	-11465 / 0	-78.0	-78.0	0.59 (1)	2.74	E-S	0 / 1058	0.14 (1)

	J-K	-6287.0	-91.0	-91.0	0.00 (1)	0.00	P-K	0 / 5811	0.74 (1)	END JACK TYPE: CONVENTIONAL
	K-L	-2700.0	-78.0	-78.0	0.26 (1)	3.94				

[illegible]

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292661	H19A	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Feb 19 12:31:38 2018 Page 2
ID:Wr0cqTsRngk4toJDtjrg3lyNlLr-9GNBS9b0AJL5u182p3iOisXF7mQXRrfyughYEJzjb 3

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
R	BMWW-t	MT20	7.0	8.0	3.75	3.00
S	BMWWW-t	MT20	6.0	9.0	3.50	4.50
T	BMWW-t	MT20	7.0	8.0	3.75	3.00
U	BS-t	MT18HS	5.0	12.0		
V	BMWW-t	MT20	6.0	9.0	2.25	2.75
W	BMWW-t	MT20	5.0	8.0	4.25	1.75
X	BMV1-t	MT20	6.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 27.5 lbs FACTORED DOWN AT 39-0-4
ON TOP CHORD, AND 1081.4 lbs FACTORED
DOWN AT 4-9-6, AND 1081.4 lbs FACTORED
DOWN AT 14-2-8 ON BOTTOM CHORD. DESIGN
FOR UNSPECIFIED CONNECTION(S) IS
DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
K	39-0-4	-27	-27	—	FRONT	VERT	TOTAL
T	14-2-8	-1081	-1081	—	FRONT	VERT	TOTAL
Z	4-9-6	-1081	-1081	—	FRONT	VERT	TOTAL

- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT
OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F.
RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED
ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.36")
CALCULATED VERT. DEFL.(LL) = L/ 998 (0.49")
ALLOWABLE DEFL.(TL)= L/360 (1.36")
CALCULATED VERT. DEFL.(TL) = L/ 506 (0.97")

CSI: TC=0.60/1.00 (B-X:1), BC=0.79/1.00 (S-T:1),
WB=0.80/1.00 (C-V:1), SSI=0.68/1.00 (V-W:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN
THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

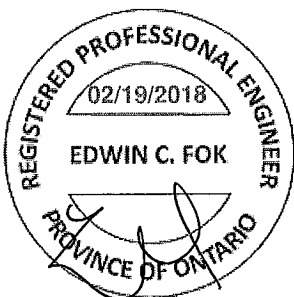
PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	618	354	1667
MT18HS	511	354	2455

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (V) (INPUT = 0.90)
JSI METAL= 0.84 (P) (INPUT = 1.00)

NOTE: Lateral brace(s) shown shall be 1x4
for Part 9 design as per OBC 9.23.13.11,
and no less than 2x4 for Part 4 design

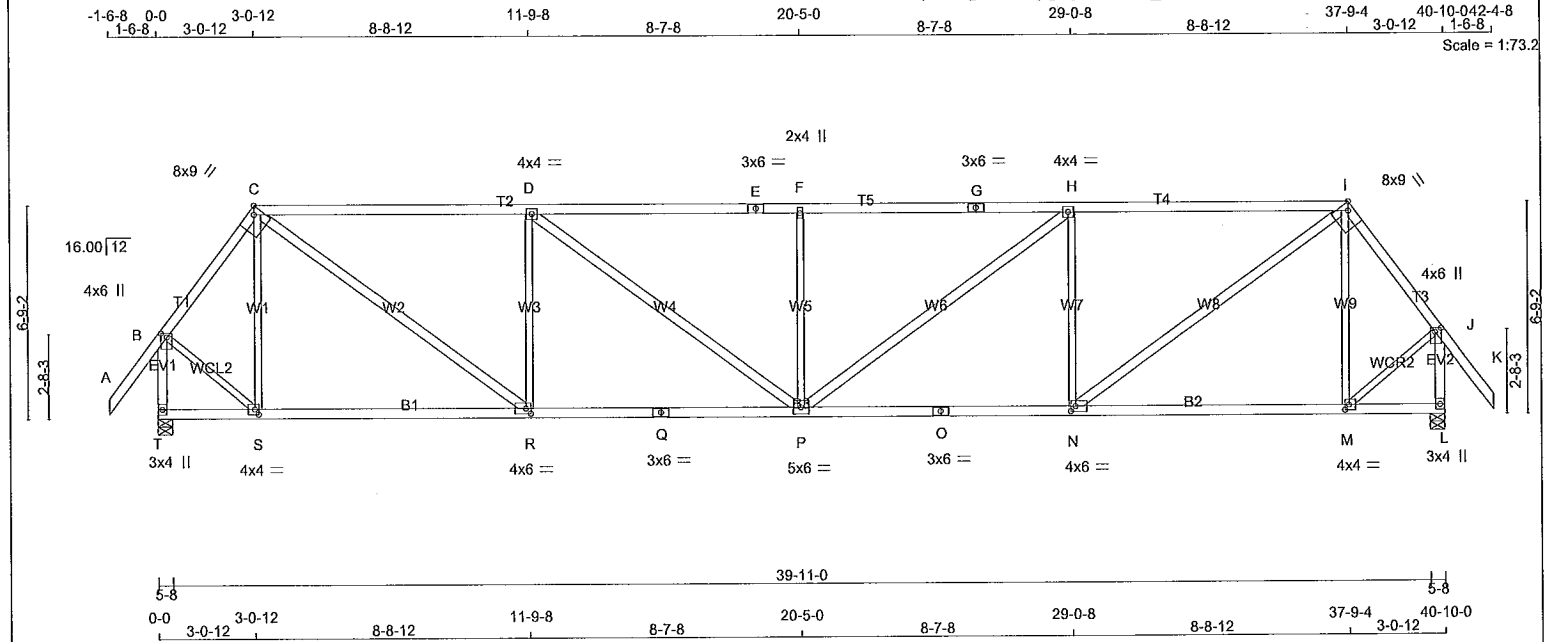


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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292661	H20	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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ID:Wr0cqTsRngk4toJDtjrg3lyNlR-FeZ_JfK1fdNHgiX6LQSR68TYVL970eVhgbeezjb7S



TOTAL WEIGHT = 188 lb

LUMBER	N. L. G. A. RULES	SIZE	LUMBER	DESCR.
A - C	2x4 DRY	No.2	SPF	
C - E	2x4 DRY	2100F 1.8E	SPF	
E - G	2x4 DRY	2100F 1.8E	SPF	
G - I	2x4 DRY	2100F 1.8E	SPF	
I - K	2x4 DRY	No.2	SPF	
T - B	2x4 DRY	No.2	SPF	
L - J	2x4 DRY	No.2	SPF	
T - Q	2x4 DRY	No.2	SPF	
Q - O	2x4 DRY	No.2	SPF	
O - L	2x4 DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3 DRY	No.2	SPF	
C - R	2x4 DRY	No.2	SPF	
D - P	2x4 DRY	No.2	SPF	
P - H	2x4 DRY	No.2	SPF	
N - I	2x4 DRY	No.2	SPF	

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	DOWN	UP	IN-SX
T	2099	0	0	5-8
L	2099	0	0	5-8

UNFACTORED REACTIONS

1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	1494	927 / 0	0 / 0	0 / 0	567 / 0	0 / 0
L	1494	927 / 0	0 / 0	0 / 0	567 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.90 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED HORIZ. LOAD (PLF)	MAX. FACTORED TORSION (PLF)	MAX. FACTORED UNBRACED LENGTH (FT)
FR-TO						
A-B	0 / 52	-78.0	-78.0	0.15 (1)	10.00	
B-C	-1498 / 0	-78.0	-78.0	0.17 (1)	5.21	
C-D	-2665 / 0	-78.0	-78.0	0.85 (1)	4.23	
D-E	-3185 / 0	-78.0	-78.0	0.93 (1)	3.90	
E-F	-3185 / 0	-78.0	-78.0	0.93 (1)	3.90	
F-G	-3185 / 0	-78.0	-78.0	0.93 (1)	3.90	
G-H	-3185 / 0	-78.0	-78.0	0.93 (1)	3.90	
H-I	-2665 / 0	-78.0	-78.0	0.85 (1)	4.23	
I-J	-1498 / 0	-78.0	-78.0	0.17 (1)	5.21	
J-K	0 / 52	-78.0	-78.0	0.15 (1)	10.00	
T-B	-2099 / 0	0.0	0.0	0.30 (1)	5.84	
L-J	-2099 / 0	0.0	0.0	0.30 (1)	5.84	
T-S	0 / 0	-18.5	-18.5	0.21 (4)	10.00	
S-R	0 / 884	-18.5	-18.5	0.37 (4)	10.00	
R-Q	0 / 2666	-18.5	-18.5	0.58 (1)	10.00	
Q-P	0 / 2666	-18.5	-18.5	0.58 (1)	10.00	
P-O	0 / 2666	-18.5	-18.5	0.58 (1)	10.00	
O-N	0 / 2666	-18.5	-18.5	0.58 (1)	10.00	
N-M	0 / 884	-18.5	-18.5	0.37 (4)	10.00	
M-L	0 / 0	-18.5	-18.5	0.21 (4)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 21.0	PSF
DL = 6.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.4	PSF	
TOTAL LOAD	= 34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.36")
CALCULATED VERT. DEFL.(LL) = L/999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (1.36")
CALCULATED VERT. DEFL.(TL) = L/999 (0.35")

CSI: TC=0.93/1.00 (D-F:1), BC=0.58/1.00 (P-R:1),
WB=0.88/1.00 (D-R:1), SS=0.32/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

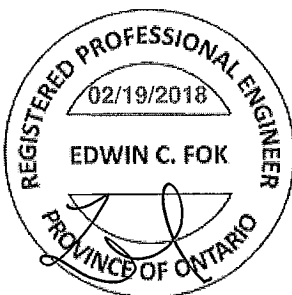
NAIL VALUES	PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 788	1987 1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (R) (INPUT = 0.90)
JSI METAL= 0.92 (O) (INPUT = 1.00)

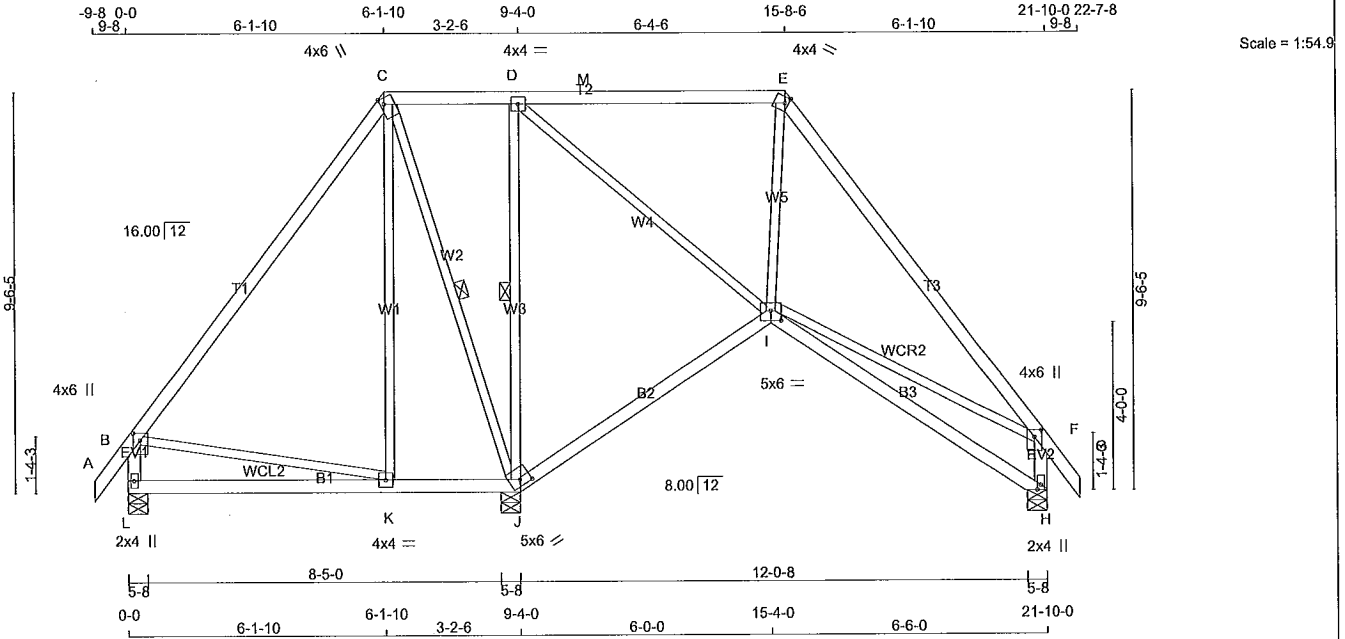
NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292661	H21T	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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ID:Wr0cqTsRngk4toJDtjrg3lyNlLr-CICAJ9YyEV1g3bFBrFv67Rq_9w64hxI37ilp2zjb79



TOTAL WEIGHT = 2 X 112 = 223 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	2100F 1.8E	SPF	
C - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	2100F 1.8E	SPF	
L - B	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
L - J	2x4	DRY	No.2	SPF	
J - I	2x4	DRY	No.2	SPF	
I - H	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TTWW+m	MT20	4.0	6.0	1.75	1.00
D	MTWW-t	MT20	4.0	4.0		
E	TTW-m	MT20	4.0	4.0	1.75	1.00
F	TMVW+p	MT20	4.0	6.0	2.00	2.00
H	BMV1+p	MT20	2.0	4.0	Edge	
I	BBWWW-p	MT20	5.0	6.0	2.75	3.00
J	BBWW1-h	MT20	5.0	6.0	1.75	3.00
K	BMWW-t	MT20	4.0	4.0		
L	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD IN-SX
	VERT	HORZ	DOWN	HORZ		
L	382	0	382	0	5-8	1-8
J	1358	0	1358	0	5-8	1-14
H	579	0	579	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS											
		SNOW		LIVE		PERM.LIVE		WIND		DEAD		SC	
L	271	174 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	97 / 0	0 / 0	0 / 0	0 / 0
J	974	563 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	411 / 0	0 / 0	0 / 0	0 / 0
H	412	256 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	156 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, J, H

BRACING

FOR SECTION C-E, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-J, D-J.

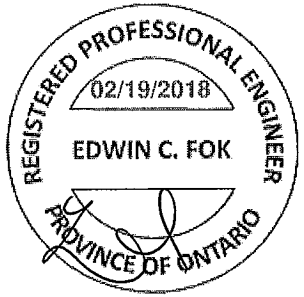
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED	FACTORED			MEMB.	MAX. FACTORED			
	FORCE (LBS)	VERT. LOAD	LC1	MAX. UNBRAC		FORCE (LBS)	MAX. CSI (LC)		
FR-TO		(PLF)		LENGTH	FR-TO				
A-B	0 / 28	-78.0	-78.0	0.03 (1)	10.00	K-C	0 / 130	0.04 (4)	
B-C	-27 / 0	-78.0	-78.0	0.25 (1)	6.25	C-J	-375 / 0	0.25 (1)	
C-D	0 / 108	-85.5	-85.5	0.47 (1)	2.00	J-D	-863 / 0	0.52 (1)	
D-M	-230 / 0	-85.5	-85.5	0.41 (1)	2.00	D-I	0 / 455	0.10 (1)	
M-E	-230 / 0	-85.5	-85.5	0.41 (1)	2.00	I-E	-144 / 12	0.07 (1)	
E-F	-402 / 0	-78.0	-78.0	0.25 (1)	6.25	B-K	0 / 16	0.00 (1)	
F-G	0 / 28	-78.0	-78.0	0.03 (1)	10.00	I-F	0 / 266	0.06 (1)	
L-B	-334 / 0	0.0	0.0	0.03 (1)	7.81				
H-F	-519 / 0	0.0	0.0	0.06 (1)	7.81				
L-K	0 / 0	-18.5	-18.5	0.15 (4)	10.00				
K-J	0 / 17	-18.5	-18.5	0.15 (4)	10.00				
J-I	-139 / 0	-18.5	-18.5	0.15 (4)	6.25				
I-H	0 / 0	-18.5	-18.5	0.23 (4)	10.00				

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	34.4	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.42")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.42")
CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CSI: TC=0.47/1.00 (C-D:1), BC=0.23/1.00 (H-I:4), WB=0.52/1.00 (D-J:1), SSI=0.26/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX MIN
MT20	618	354	1687 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

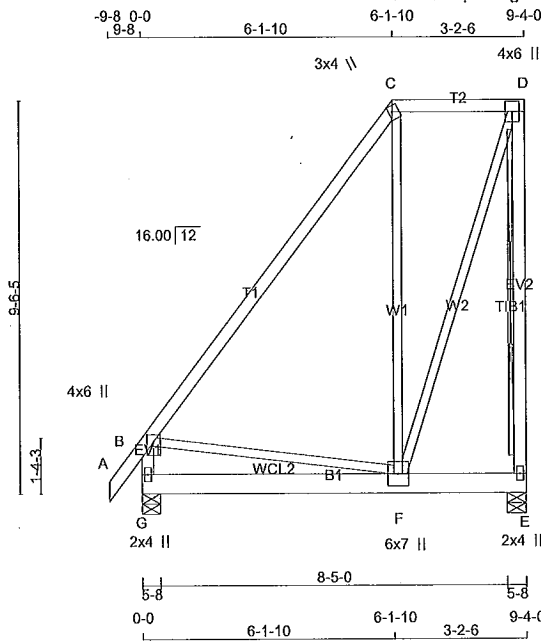
JSI GRIP= 0.82 (J) (INPUT = 0.90)
JSI METAL= 0.32 (J) (INPUT = 1.00)

A-18023965

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292661	H22S	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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Scale = 1:56.0

TOTAL WEIGHT = 64 lb

LUMBER	CHORDS	SIZE	DRY	LUMBER	DESCR.
N. L. G. A. RULES					
A - C	2x4	DRY	No.2	2100F 1.8E	SPF
C - D	2x4	DRY	No.2		SPF
E - D	2x4	DRY	No.2		SPF
G - B	2x4	DRY	No.2		SPF
G - E	2x6	DRY	No.2		SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TTW+m	MT20	3.0	4.0	1.75	1.00
D	TMVW+p	MT20	4.0	6.0		
E	BMV1+p	MT20	2.0	4.0		
F	BMVWW+t	MT20	6.0	7.0		
G	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
E	1388	0	1388	0
G	1459	0	1459	0

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	990	604 / 0	0 / 0	0 / 0	0 / 0	386 / 0	0 / 0
G	1039	642 / 0	0 / 0	0 / 0	0 / 0	397 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT D-E

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX	CS1 (LC)	UNBRAC LENGTH	FR-TO	WEBS	MAX. FACTORED FORCE (LBS)	MAX CS1 (LC)
FR-TO			FROM	TO							
A-B		0 / 28	-78.0	-78.0	0.03	(1)	10.00	F-C		0 / 148	0.05 (4)
B-C		-646 / 0	-78.0	-78.0	0.41	(1)	6.25	F-D		0 / 1233	0.31 (1)
C-D		-389 / 0	-78.0	-78.0	0.15	(1)	6.25	B-F		0 / 390	0.10 (1)
E-D		-1289 / 0	0.0	0.0	0.70	(1)	7.81				
G-B		-882 / 0	0.0	0.0	0.10	(1)	7.81				
G-F		0 / 0	-219.5	-219.5	0.56	(1)	10.00				
F-E		0 / 0	-219.5	-219.5	0.52	(1)	10.00				

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CSldGlrdr
START DISTANCE = 0-0
START SPAN CARRIED = 10-4-0
END DISTANCE = 9-4-0
END SPAN CARRIED = 10-4-0
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDTL LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.31")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.31")
CALCULATED VERT. DEFL.(TL) = L/715 (0.16")

CS1: TC=0.70/1.00 (D-E:1), BC=0.56/1.00 (F-G:1), WB=0.31/1.00 (D-F:1), SSI=0.53/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

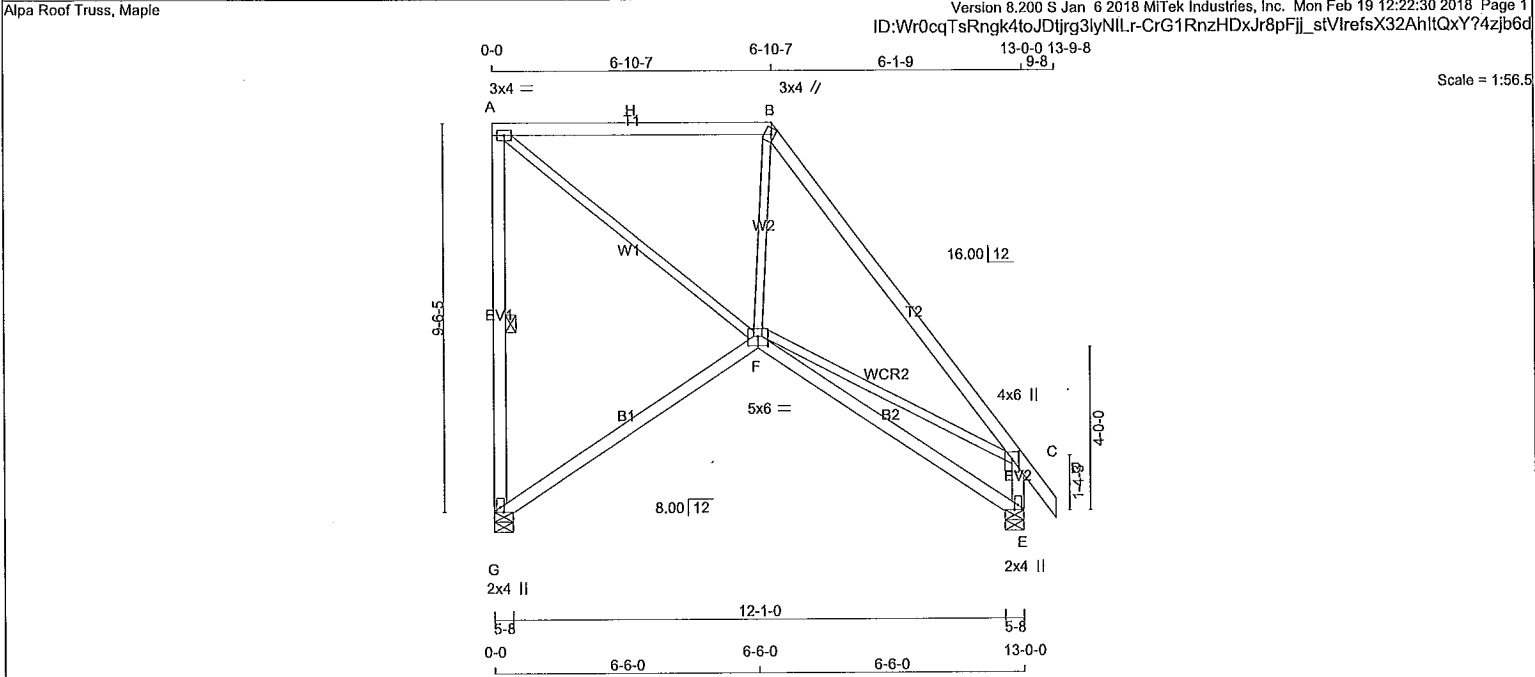
PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (D) (INPUT = 0.90)
JSI METAL= 0.28 (B) (INPUT = 1.00)

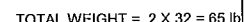
A-18023966

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292661	H23T	4	1	TRUSS DESC.	



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR.						DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS						DESIGN CRITERIA					
G - A	2x4	DRY	No.2	SPF		UNFACTORED REACTIONS						SPECIFIED LOADS:					
A - B	2x4	DRY	2100F 1.8E	SPF		1ST LCASE						TOP CH. LL = 21.0 PSF					
B - D	2x4	DRY	2100F 1.8E	SPF		MAX./MIN. COMPONENT REACTIONS						DL = 6.0 PSF					
E - C	2x4	DRY	No.2	SPF		FACTORED						BOT CH. LL = 0.0 PSF					
G - F	2x4	DRY	No.2	SPF		GROSS REACTION						DL = 7.4 PSF					
F - E	2x4	DRY	No.2	SPF		VERT						TOTAL LOAD = 34.4 PSF					
ALL WEBS 2x3 DRY No.2 SPF						SNOW						SPACING = 24.0 IN. C/C					
EXCEPT						LIVE						LOADING IN FLAT SECTION BASED ON					
DRY: SEASONED LUMBER.						PERM.LIVE						PIGGYBACK TRUSS WITH SLOPES OF 6.00/12					
						WIND						AND -6.00/12 AND RESPECTIVE WALL					
						DEAD						HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL					
						SOIL						DEAD LOAD OF 3.0 P.S.F.					
						BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, E						THIS TRUSS IS DESIGNED FOR RESIDENTIAL					
						OR SMALL BUILDING REQUIREMENTS OF						PART 9, NBCC 2010, NBCC 2015					
						BRACING						THIS DESIGN COMPLIES WITH:					
						FOR SECTION A-B, MAX. PURLIN SPACING = 2.00 FT.						- PART 9 OF OBC 2012, OBC 2018					
						FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.						- CSA 086-09, CSA 086-14					
						MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.						- TPIC 2011, TPIC 2014					
						ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.						(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD					
						1 LATERAL BRACE(S) AT 1/2 LENGTH OF A-G.						ALLOWABLE DEFL.(LL)= L/360 (0.43")					
						END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW						CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")					
						LOADING						ALLOWABLE DEFL.(TL)= L/360 (0.43")					
						TOTAL LOAD CASES: (4)						CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")					
						CHORDS						CSI: TC=0.45/1.00 (A-B:1) , BC=0.23/1.00 (F-G:4) ,					
						MEMB. MAX. FACTORED FORCE (LBS)						WB=0.11/1.00 (A-F:1) , SS=0.23/1.00 (A-B:1)					
						VERT. LOAD LC1 MAX (PLF)						DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10					
						FROM TO						COMP=1.10 SHEAR=1.10 TENS= 1.10					
						MAX. UNBRACED LENGTH FR-TO						COMPANION LIVE LOAD FACTOR = 1.00					
						MEMB. MAX. FACTORED FORCE (LBS)						TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .					
						CSI (LC)						NAIL VALUES					
						G-A -605 / 0						PLATE GRIP(DRY) SHEAR SECTION					
						A-H -392 / 0						(PSI) (PLI) (PLI)					
						H-B -392 / 0						MAX MIN MAX MIN MAX MIN					
						B-C -657 / 0						MT20 618 354 1667 788 1987 1656					
						C-D 0 / 28						PLATE PLACEMENT TOL. = 0.250 inches					
						E-C -651 / 0						PLATE ROTATION TOL. = 5.0 Deg.					
						G-F 0 / 0						JSI GRIP= 0.83 (E) (INPUT = 0.90)					
						F-E 0 / 0						JSI METAL= 0.32 (E) (INPUT = 1.00)					
												A-18023967					

Alpa Roof Truss, Maple



CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED			
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 10	-78.0	-78.0 0.01 (1)	10.00	F-C	-152 / 0	0.09 (1)
B-C	-114 / 0	-78.0	-78.0 0.25 (1)	6.25			
C-D	-114 / 0	-78.0	-78.0 0.25 (1)	6.25			
D-E	0 / 10	-78.0	-78.0 0.01 (1)	10.00			
B-F	0 / 68	-18.5	-18.5 0.10 (4)	10.00			
F-D	0 / 68	-18.5	-18.5 0.10 (4)	10.00			

A-18023968

Alpa Roof Truss, Maple

Scale = 1:24.4

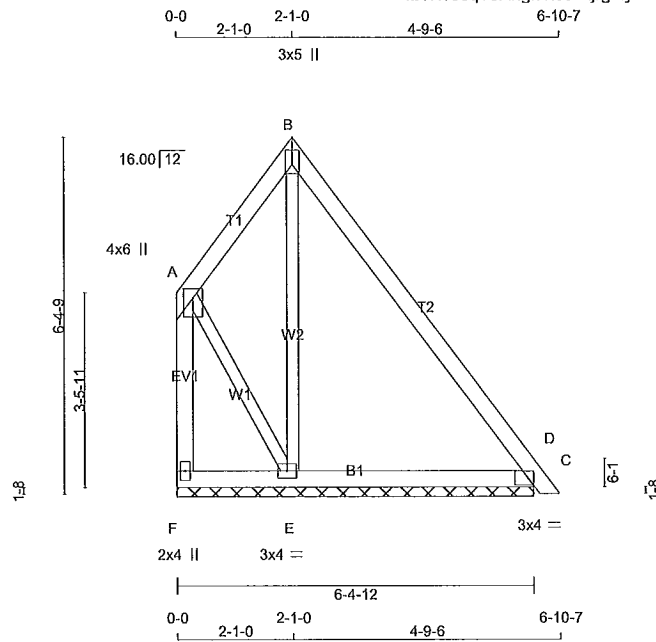


A-18023969

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292661	H26P	4	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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Scale = 1:41.2

TOTAL WEIGHT = 4 X 32 = 127 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF	
A - B	2x4	DRY	No.2	SPF	
B - D	2x4	DRY	No.2	SPF	
F - C	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A - TMVW+p	MT20	4.0	6.0	2.00	2.00
B - TTW+p	MT20	3.0	5.0	2.00	Edge
C - TMB1-l	MT20	3.0	4.0	1.50	2.75
E - BMVW1-l	MT20	3.0	4.0		
F - BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
GROSS REACTION	GROSS REACTION	DOWN	HORZ	UPLIFT
VERT	HORZ	DOWN	HORZ	UPLIFT
JT				
F	185	0	185	0
C	276	0	276	0
E	183	0	183	0

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	128	98 / 0	0 / 0	0 / 0	0 / 0	31 / 0	0 / 0
C	195	131 / 0	0 / 0	0 / 0	0 / 0	63 / 0	0 / 0
E	136	54 / 0	0 / 0	0 / 0	0 / 0	81 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			FR-TO		
F-A	-181 / 0	0.0	0.0	0.04 (1)	7.81	A-E	0 / 63
A-B	-59 / 0	-78.0	-78.0	0.06 (1)	6.25	E-B	-154 / 0
B-C	-60 / 0	-78.0	-78.0	0.25 (1)	6.25		
C-D	0 / 10	-78.0	-78.0	0.01 (1)	10.00		
F-E	0 / 0	-18.5	-18.5	0.08 (4)	10.00		
E-C	0 / 35	-18.5	-18.5	0.08 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.25/1.00 (B-C:1), BC=0.08/1.00 (C-E:4), WB=0.09/1.00 (B-E:1), SSI=0.08/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

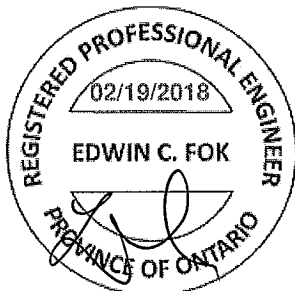
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.31 (C) (INPUT = 0.90)
JSI METAL= 0.11 (C) (INPUT = 1.00)

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

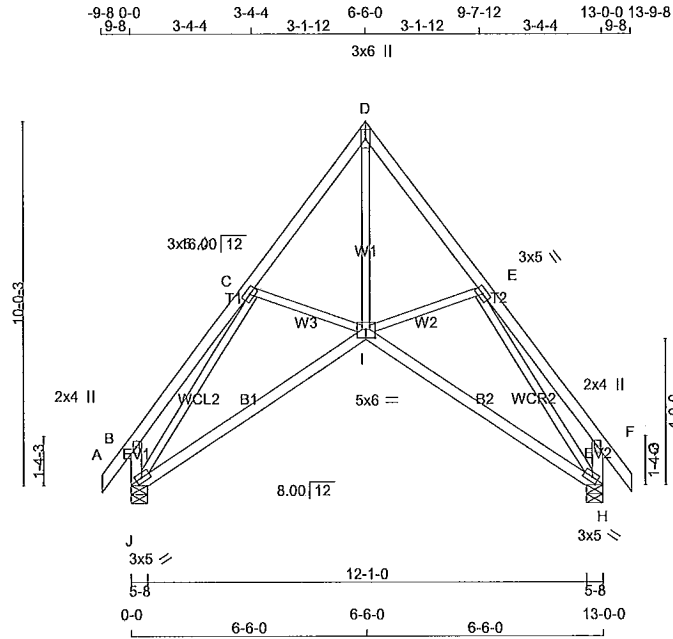


A-18023970

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292661	H27T	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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Scale: 3/16"=1'

TOTAL WEIGHT = 2 X 68 = 137 lb
(M)(F)

LUMBER					DESCR.
N. L. G. A. RULES	CHORDS	SIZE	LUMBER		
A - D	2x4	DRY	No.2	SPF	
D - G	2x4	DRY	No.2	SPF	
J - B	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
J - I	2x4	DRY	No.2	SPF	
I - H	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	2.0	4.0	
C	TMVW-t	MT20	3.0	5.0	
D	TTW+p	MT20	3.0	6.0	
E	TMVW-t	MT20	3.0	5.0	
F	TMV+p	MT20	2.0	4.0	
H	BMVW1-t	MT20	3.0	5.0	1.50 2.00
I	BBVWV-p	MT20	5.0	6.0	2.75 3.00
J	BMVW1-t	MT20	3.0	5.0	1.50 2.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQRD
JT	GROSS REACTION	GROSS REACTION	DOWN	BRG	IN-SX
J	698	0	698	0	5-8
H	698	0	698	0	5-8

UNFACTORED REACTIONS		1ST LCASE	MAX./MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
J	496	311 / 0	0 / 0
H	496	311 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING		TOTAL LOAD CASES: (4)			
CHORDS		MAX. FACTORED		WEBS	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	MEMB.	FORCE (LBS)
FR-TO		FROM TO	CS1 (LC)	LENGTH FR-TO	MAX CS1 (LC)
A-B	0 / 28	-78.0 -78.0	0.05 (1)	I-D	0 / 769
B-C	0 / 21	-78.0 -78.0	0.12 (1)	I-E	-83 / 0
C-D	-628 / 0	-78.0 -78.0	0.10 (1)	C-I	-83 / 0
D-E	-628 / 0	-78.0 -78.0	0.10 (1)	J-C	-881 / 0
E-F	0 / 21	-78.0 -78.0	0.12 (1)	E-H	-881 / 0
F-G	0 / 28	-78.0 -78.0	0.05 (1)		
J-B	-170 / 0	0.0 0.0	0.02 (1)		
H-F	-170 / 0	0.0 0.0	0.02 (1)		
J-I	0 / 531	-18.5 -18.5	0.27 (4)		
I-H	0 / 531	-18.5 -18.5	0.27 (4)		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.12")

CSI: TC=0.12/1.00 (B-C:1), BC=0.27/1.00 (I-J:4), WB=0.60/1.00 (C-J:1), SSI=0.07/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

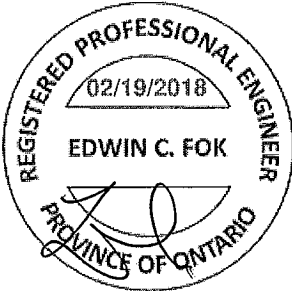
NAIL VALUES		PLATE GRIP(DRY) SHEAR		SECTION	
(PSI)	(PLI)	(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (C) (INPUT = 0.90)
JSI METAL= 0.27 (J) (INPUT = 1.00)

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

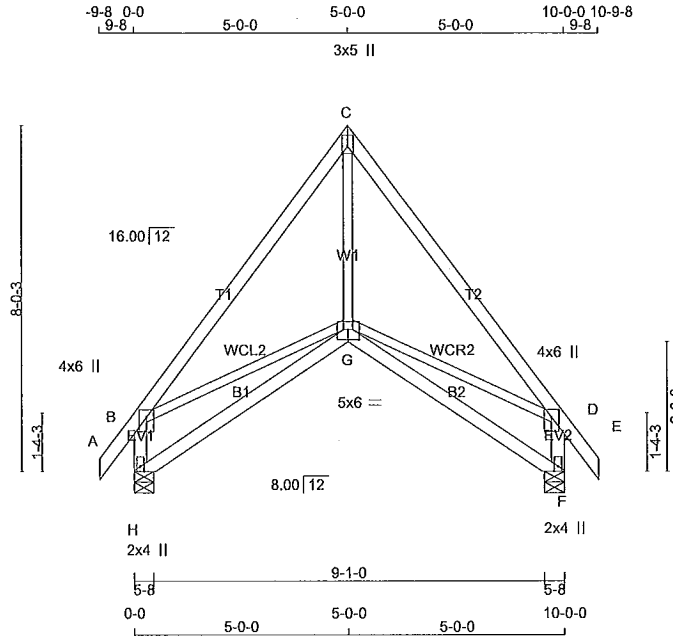


A-18023971

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292661	H28T	4	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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ID:Wr0cqTsRngk4toJDtjrg3lyNlR-piKkw4nMu1CZ8sjYVUZ3aw2GFKkvB08NNIHUGNzb4H



TOTAL WEIGHT = 4 X 51 = 205 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
H - B	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	
H - G	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TTW+p	MT20	3.0	5.0	2.00	Edge
D	TMVW+p	MT20	4.0	6.0	2.00	2.00
F	BMV1+p	MT20	2.0	4.0	Edge	
G	BBWW+p	MT20	5.0	6.0	2.75	3.00
H	BMV1+p	MT20	2.0	4.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
H	659	0	659	0	5-8	1-8
F	659	0	659	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	468	294 / 0	0 / 0	0 / 0	0 / 0	174 / 0	0 / 0
F	468	294 / 0	0 / 0	0 / 0	0 / 0	174 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM	TO			FR-TO			
A- B	0 / 28	-78.0	-78.0	0.05 (1)	10.00	G- C	0 / 377	0.09 (1)	
B- C	-533 / 0	-95.0	-95.0	0.49 (1)	6.25	B- G	0 / 346	0.09 (1)	
C- D	-533 / 0	-95.0	-95.0	0.49 (1)	6.25	G- D	0 / 346	0.09 (1)	
D- E	0 / 28	-78.0	-78.0	0.05 (1)	10.00				
H- B	-602 / 0	0.0	0.0	0.07 (1)	7.81				
F- D	-602 / 0	0.0	0.0	0.07 (1)	7.81				
H- G	0 / 0	-22.5	-22.5	0.18 (4)	10.00				
G- F	0 / 0	-22.5	-22.5	0.18 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CPrimeHlp
SIDE SETBACK = 0-0
END SETBACK = 2-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADD'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.33")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.33")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.49/1.00 (C-D:1), BC=0.18/1.00 (F-G:4), WB=0.09/1.00 (C-G:1), SSI=0.12/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

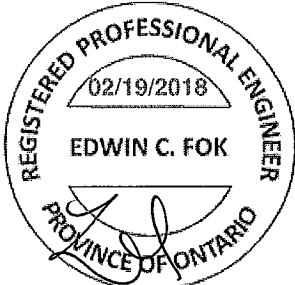
PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)	(PLI)	(PSI)	(PLI)	(PSI)	(PLI)
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (F) (INPUT = 0.90)
JSI METAL= 0.30 (F) (INPUT = 1.00)

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

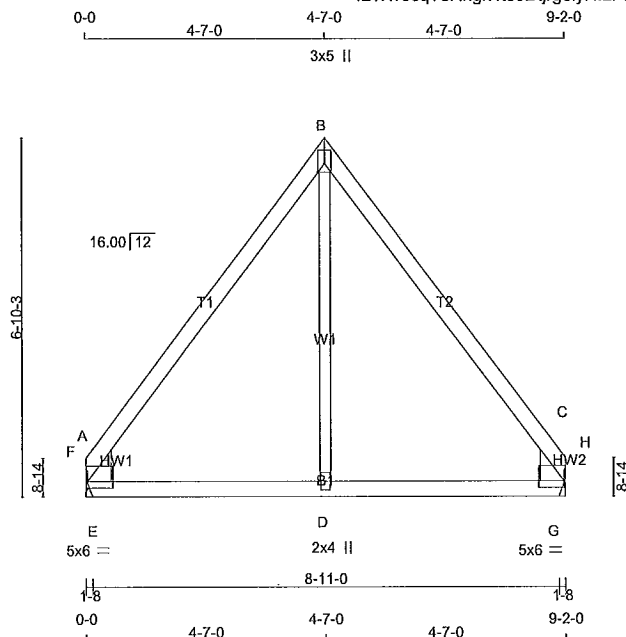


A-18023972

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292661	H29	4	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Feb 19 12:24:30 2018 Page 1
ID:Wr0cqTsRngk4toJDjrg3lyNlR-hxW0dpQYzPQqYFQnlB00MbpBOEUQ9SJzT50uCEzjb4f



Scale = 1:44.0

TOTAL WEIGHT = 4 X 36 = 143 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF	
B - C	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
A	TMBH1-I	MT20	5.0	6.0	1.50	0.25	
B	TTW+p	MT20	3.0	5.0	2.00	Edge	
C	TMBH1-I	MT20	5.0	6.0	1.50	0.25	
D	BMW+w	MT20	2.0	4.0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG	HEEL
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
A	539	0	539	0	0
C	539	0	539	0	0

UNFACTORED REACTIONS

JT	1ST CASE	MAX./MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	384	234 / 0	0 / 0	0 / 0	0 / 0	0 / 0	150 / 0	0 / 0
C	384	234 / 0	0 / 0	0 / 0	0 / 0	0 / 0	150 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX	CS1 (LC)	UNBRAC LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CS1 (LC)
FR-TO											
A - F	-163 / 17	-95.0	-95.0	0.21 (1)	6.25	D - B	0 / 204	0.06 (4)			
F - B	-374 / 0	-95.0	-95.0	0.26 (1)	6.25	E - F	-611 / 0	0.00 (1)			
B - H	-374 / 0	-95.0	-95.0	0.26 (1)	6.25	G - H	-611 / 0	0.00 (1)			
H - C	-163 / 17	-95.0	-95.0	0.21 (1)	6.25						
A - E	0 / 209	-22.5	-22.5	0.21 (1)	10.00						
E - D	0 / 209	-22.5	-22.5	0.21 (1)	10.00						
D - G	0 / 209	-22.5	-22.5	0.21 (1)	10.00						
G - C	0 / 209	-22.5	-22.5	0.21 (1)	10.00						

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 0-0
END SETBACK = 2-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADD'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.31")
CALCULATED VERT. DEFL.(LL)= L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.31")
CALCULATED VERT. DEFL.(TL)= L/999 (0.03")

CSI: TC=0.26/1.00 (B-H:1) , BC=0.21/1.00 (D-E:1) , WB=0.06/1.00 (B-D:4) , SSI=0.53/1.00 (C-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (A) (INPUT = 0.90)
JSI METAL= 0.22 (A) (INPUT = 1.00)

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



A-18023973

PLATE ROTATION TOL. = 5.0 Deg.
A-18023974 CONTINUED ON PAGE 8

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292661	H30A	2	2	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 8.200 S Jan 6 2018 MITek Industries, Inc. Mon Feb 19 12:26:16 2018 Page 2
ID:Wr0cqTsRngk4toJDtjrg3lyNlLr-aKwTt3i1?NnnGYX9yRwtfyCM?zbM?KFZrahVxzzjb35

PLATES (table is in inches)

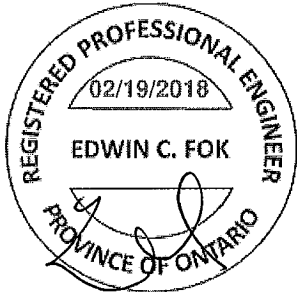
JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	9.0	2.00	3.00
B	TTWW-m	MT20	4.0	6.0	2.00	1.00
C	TMVW-t	MT20	4.0	6.0	2.00	2.75
D	TMV+p	MT20	2.0	4.0		
E	BMVW1-t	MT20	3.0	5.0	1.50	2.00
F	BBVW-I	MT20	6.0	7.0		
G	BMVW-t	MT20	3.0	5.0	1.50	1.50
H	BVM1-I	MT20	3.0	6.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 538.6 lbs FACTORED DOWN AT 2-9-0
ON BOTTOM CHORD. DESIGN FOR
UNSPECIFIED CONNECTION(S) IS DELEGATED
TO THE BUILDING DESIGNER.

JSI GRIP= 0.86 (C) (INPUT = 0.90)
JSI METAL= 0.33 (C) (INPUT = 1.00)

NOTE: Lateral brace(s) shown shall be 1x4
for Part 9 design as per OBC 9.23.13.11,
and no less than 2x4 for Part 4 design

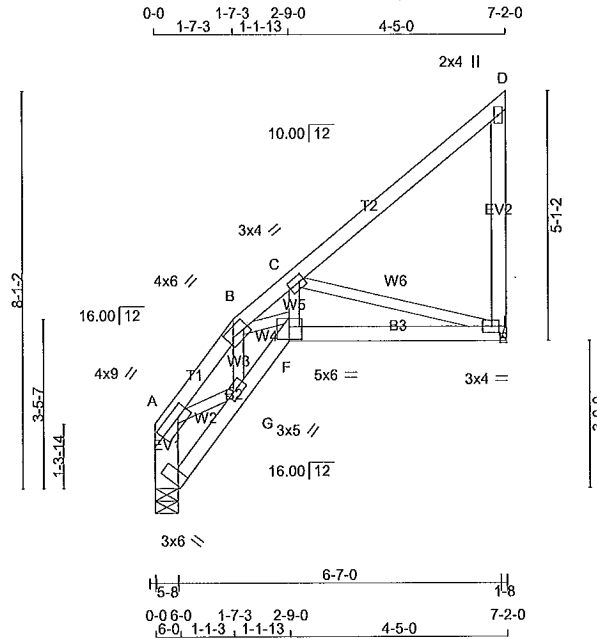


A:18023974(2)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292661	H30T	5	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Feb 19 12:26:27 2018 Page 1
ID:Wr0cqTsRngk4toJDtjrg3lyNlR-mR4dBqqxPmAD4EIG5FdSbG9DJPRY4KvBNorbpqzb2w



TOTAL WEIGHT = 5 X 36 = 182 lb
[M][F]

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
H - A	2x6 DRY	No.2	SPF		
A - B	2x4 DRY	No.2	SPF		
B - D	2x4 DRY	No.2	SPF		
E - D	2x4 DRY	No.2	SPF		
H - F	2x4 DRY	No.2	SPF		
F - E	2x4 DRY	No.2	SPF		
ALL WEBS	2x3 DRY	No.2	SPF		
DRY:	SEASONED LUMBER.				

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	9.0	2.00	3.00	
B	TTWW-m	MT20	4.0	6.0	2.00	1.50	
C	TMVW-t	MT20	3.0	4.0	1.50	1.25	
D	TMV+p	MT20	2.0	4.0			
E	BMVW-t	MT20	3.0	4.0	1.50	1.75	
F	BBWW-t	MT20	5.0	6.0	3.00	3.00	
G	BMVW-t	MT20	3.0	5.0	1.50	1.50	
H	BVM1+t	MT20	3.0	6.0	Edge		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQRD
JT	VERT	HORZ	DOWN	HORZ
H	346	0	346	0
E	346	0	346	0
			5-8	1-8
			HANGER BY OTHERS	
			MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	COMBINED						
H	247	150 / 0	0 / 0	0 / 0	0 / 0	96 / 0	0 / 0
E	247	150 / 0	0 / 0	0 / 0	0 / 0	96 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H
BEARING SIZE FACTOR = 1.15 AT JNT(S) H (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MEMB.	FORCE (LBS)
FR-TO		FROM TO	FR-TO	
H-A	-328 / 0	0.0 0.0 0.02 (1)	A-G	0 / 325
A-B	-498 / 0	-78.0 -78.0 0.03 (1)	G-B	-207 / 0
B-C	-825 / 0	-78.0 -78.0 0.13 (1)	B-F	0 / 394
C-D	-18 / 0	-78.0 -78.0 0.17 (1)	F-C	0 / 408
E-D	-140 / 0	0.0 0.0 0.06 (1)	C-E	-660 / 0
H-G	-1 / 1	-18.5 -18.5 0.01 (4)		
G-F	0 / 460	-18.5 -18.5 0.08 (1)		
F-E	0 / 637	-18.5 -18.5 0.16 (1)		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.24")
CALCULATED VERT. DEFL.(LL)= L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.24")
CALCULATED VERT. DEFL.(TL)= L/999 (0.03")

CSI: TC=0.17/1.00 (C-D:1), BC=0.16/1.00 (E-F:1), WB=0.21/1.00 (C-E:1), SSI=0.12/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

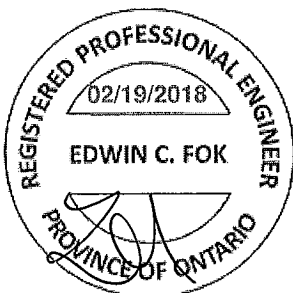
NAIL VALUES	PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	MAX MIN	
MT20	618	354	1667	788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

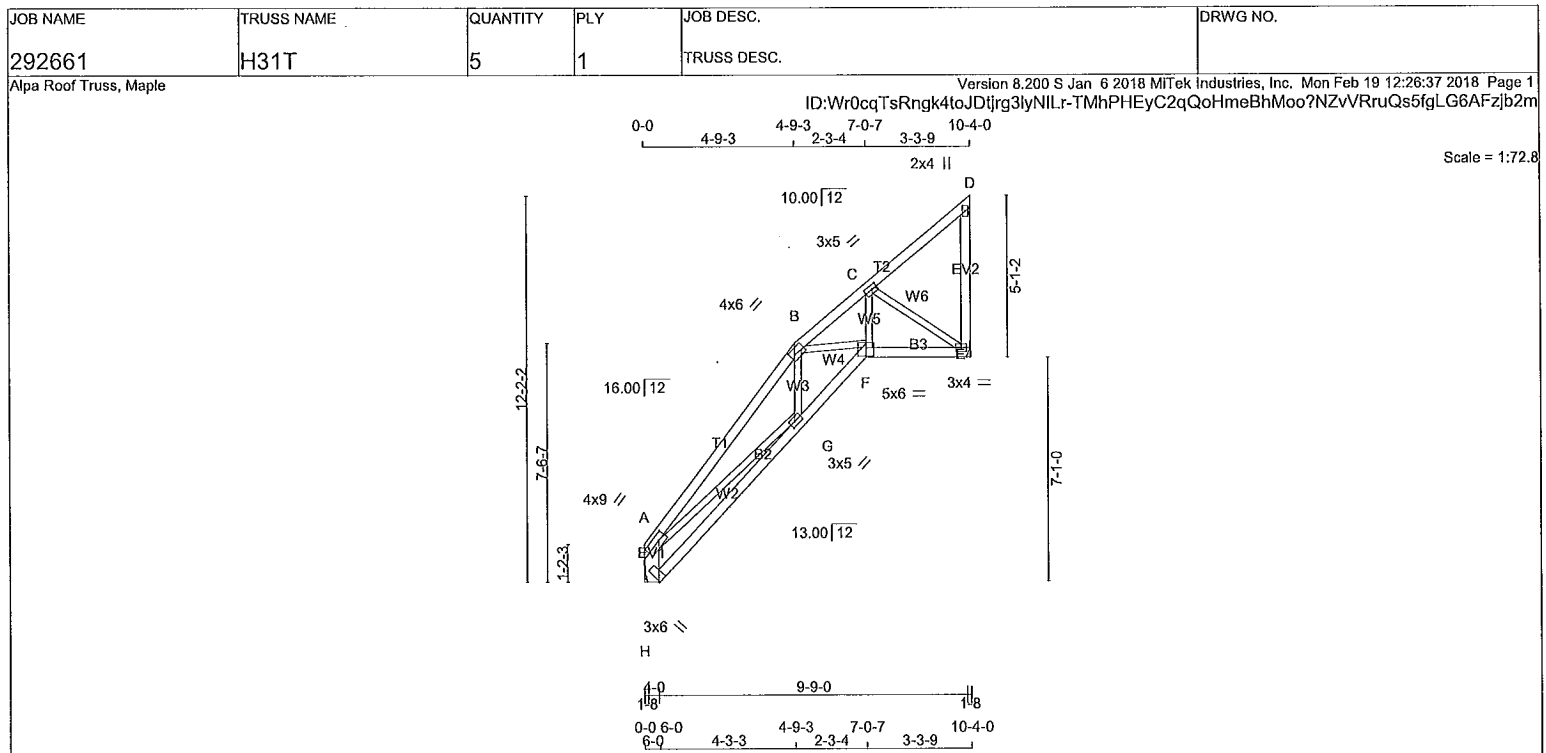
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (C) (INPUT = 0.90)
JSI METAL= 0.25 (C) (INPUT = 1.00)

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



A-18023975



TOTAL WEIGHT = 5 X 53 = 267 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
H - A	2x6	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW-t	MT20	4.0	9.0	2.00 3.00
B	TTWW-m	MT20	4.0	6.0	2.00 1.50
C	TMVW-t	MT20	3.0	5.0	1.50 2.00
D	TMV+p	MT20	2.0	4.0	
E	BMVW1-t	MT20	3.0	4.0	
F	BBWW-I	MT20	5.0	6.0	3.00 3.00
G	BMVW-t	MT20	3.0	5.0	1.50 2.00
H	BVM1-t	MT20	3.0	6.0	0.25 Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT	VERT	HORZ	DOWN	UP	UP	IN-SX	IN-SX	IN-SX	IN-SX
H	499	0	499	0	0			HANGER BY OTHERS	
E	499	0	499	0	0			HANGER BY OTHERS	

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	355	217 / 0	0 / 0	0 / 0	0 / 0	138 / 0	0 / 0
E	355	217 / 0	0 / 0	0 / 0	0 / 0	138 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.23 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. FACTORED FORCE (LBS)
FR-TO			
H-A	-458 / 0	0.0 0.0 0.03 (1)	7.81 A-G 0 / 694
A-B	-892 / 0	-78.0 -78.0 0.25 (1)	6.23 G-B -39 / 46
B-C	-780 / 0	-78.0 -78.0 0.08 (1)	6.25 B-F 0 / 65
C-D	-14 / 0	-78.0 -78.0 0.09 (1)	6.25 F-C 0 / 607
E-D	-103 / 0	0.0 0.0 0.04 (1)	7.81 C-E -676 / 0
H-G	0 / 6	-18.5 -18.5 0.10 (4)	10.00
G-F	0 / 765	-18.5 -18.5 0.16 (4)	10.00
F-E	0 / 565	-18.5 -18.5 0.12 (1)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.34")
CALCULATED VERT. DEFL.(LL)= L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.34")
CALCULATED VERT. DEFL.(TL)= L/999 (0.06")

CSI: TC=0.25/1.00 (A-B:1), BC=0.16/1.00 (F-G:4), WB=0.16/1.00 (C-E:1), SSI=0.09/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

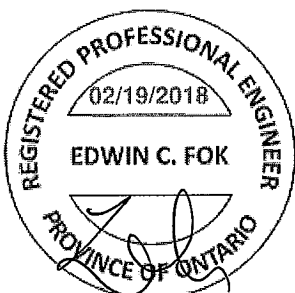
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 Inches

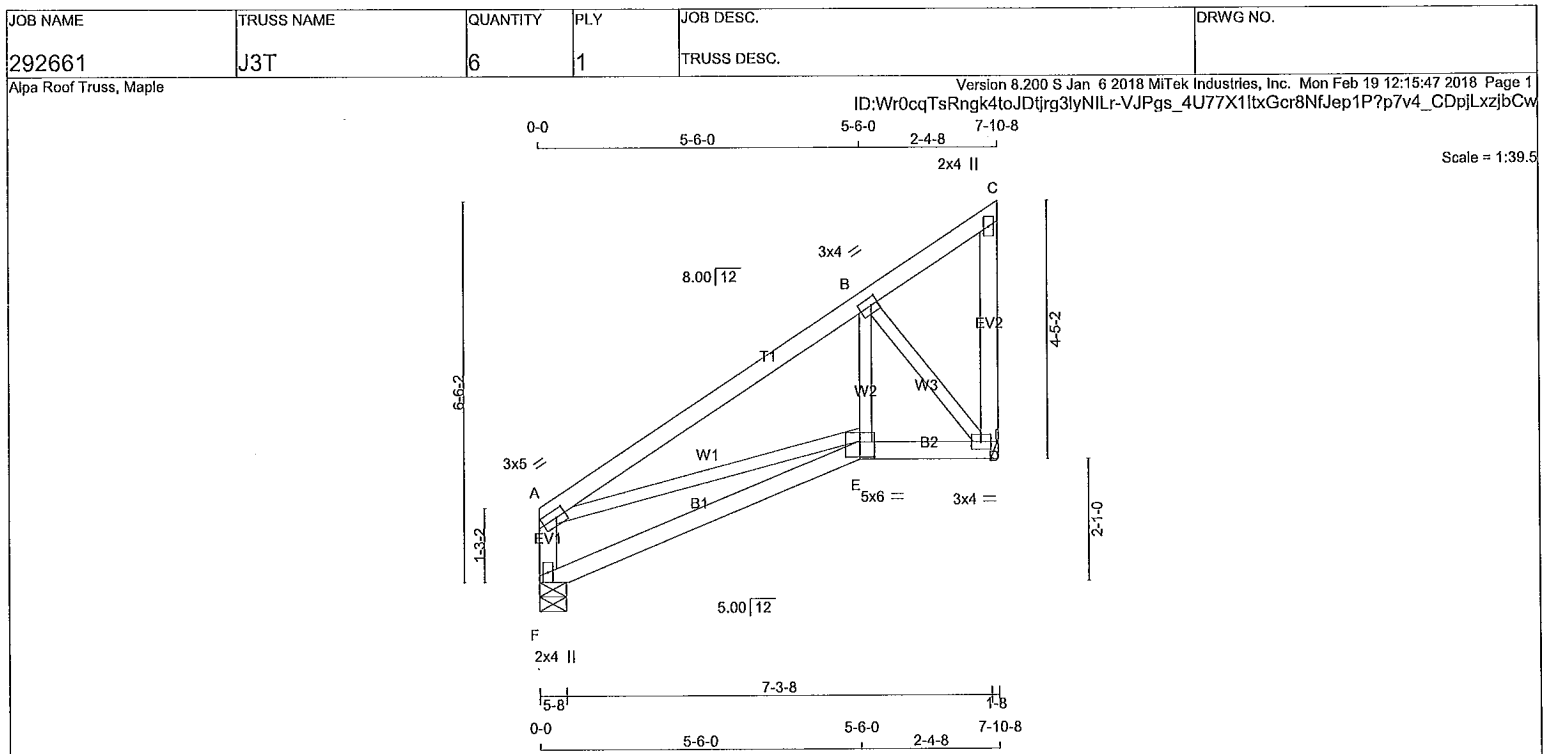
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (E) (INPUT = 0.90)
JSI METAL= 0.22 (G) (INPUT = 1.00)

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



A-18023976



TOTAL WEIGHT = 6 X 35 = 209 lb

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS 2x3 DRY No.2 SPF				
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW-t	MT20	3.0	5.0	1.50 2.00
B	TMVW-t	MT20	3.0	4.0	1.50 1.50
C	TMV+p	MT20	2.0	4.0	
D	BMVW1-t	MT20	3.0	4.0	
E	BBVW-t	MT20	5.0	6.0	3.00 3.00
F	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX	IN-SX	IN-SX
JT	380	0	380	0	5-8	1-8		
F	380	0	380	0	5-8	1-8		
D	380	0	380	0	5-8	1-8		

UNFACTORED REACTIONS

1ST CASE	MAX / MIN	COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	271	165 / 0	0 / 0	0 / 0	0 / 0	106 / 0	0 / 0
D	271	165 / 0	0 / 0	0 / 0	0 / 0	106 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		WEBS		FACTORED	
	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (PLF)	LC1 MAX (PLF)	MEMB. MAX. FORCE (LBS)	MAX. FORCE (LBS)	LC1 MAX (PLF)	LC1 MAX (PLF)
FR-TO		FROM	TO	CS (LC)	FR-TO		FROM	TO
F-A	-329 / 0	0.0	0.0	0.03 (1)	7.81	A-E	0 / 336	0.08 (1)
A-B	-373 / 0	-78.0	-78.0	0.28 (1)	6.25	E-B	0 / 142	0.04 (4)
B-C	-55 / 0	-78.0	-78.0	0.23 (1)	6.25	B-D	-490 / 0	0.10 (1)
D-C	0 / 10	0.0	0.0	0.00 (1)	10.00			
F-E	0 / 0	-18.5	-18.5	0.16 (4)	10.00			
E-D	0 / 323	-18.5	-18.5	0.07 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.26")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.26")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.28/1.00 (A-B:1), BC=0.16/1.00 (E-F:4), WB=0.10/1.00 (B-D:1), SSI=0.17/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

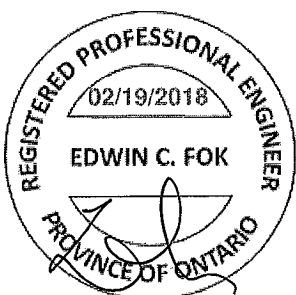
NAIL VALUES				
PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)	
	MAX	MIN	MAX	MIN
MT20	618	354	1667	788

PLATE PLACEMENT TOL. = 0.250 inches

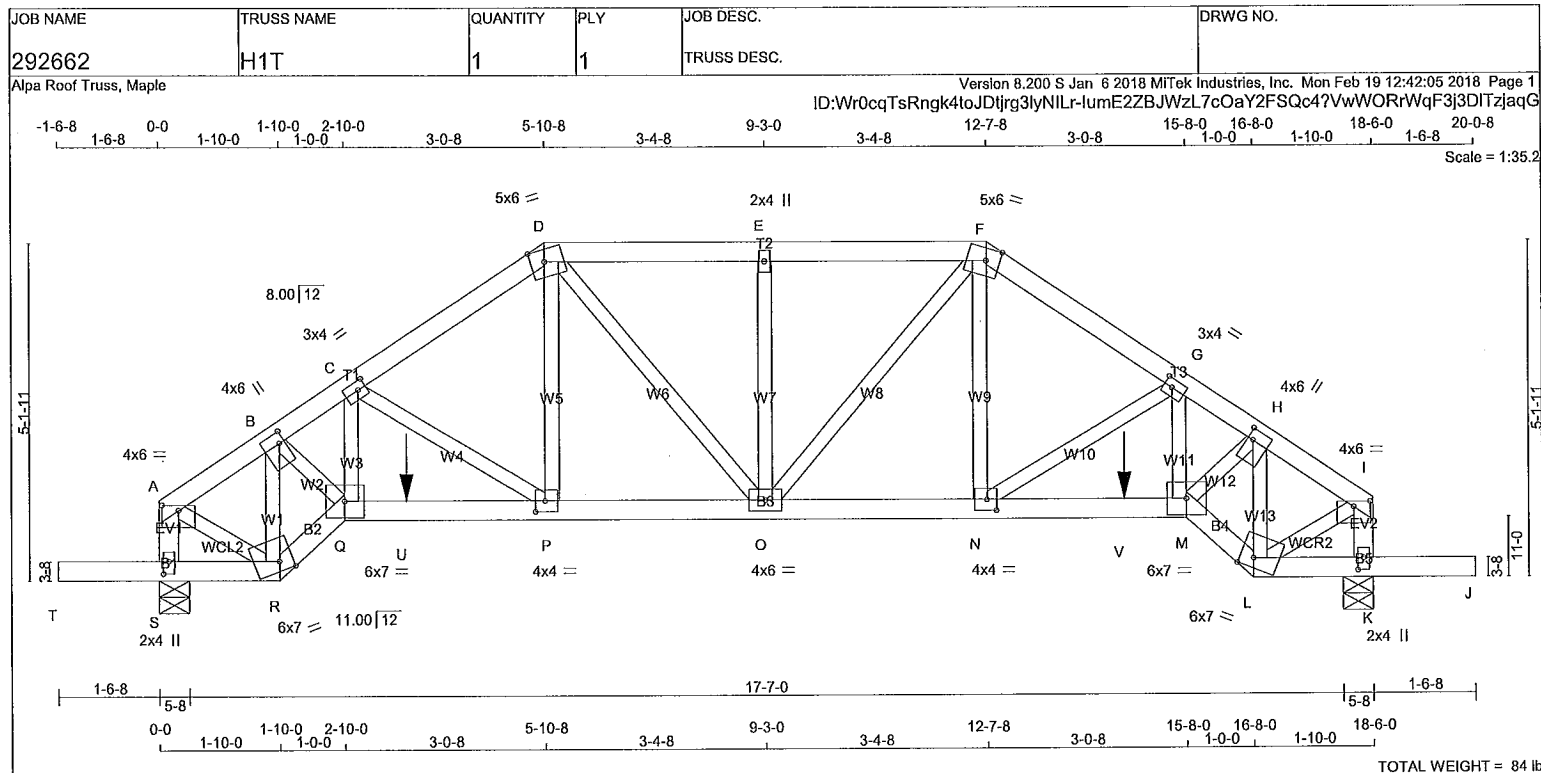
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.63 (B) (INPUT = 0.90)
JSI METAL= 0.15 (A) (INPUT = 1.00)

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



A-18023977



LUMBER	N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS				
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
S - A	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
R - Q	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

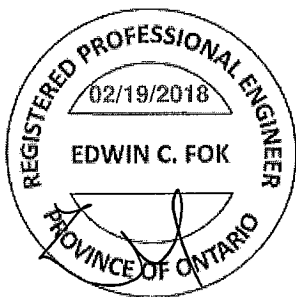
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.00	3.00
B	TMVW-t	MT20	4.0	6.0	2.00	1.00
C	TMVW-t	MT20	3.0	4.0	1.50	1.50
D	TTVW-m	MT20	5.0	6.0	Edge	2.50
E	TMVW-p	MT20	2.0	4.0		
F	TTVW-m	MT20	5.0	6.0	Edge	2.50
G	TMVW-t	MT20	3.0	4.0	1.50	1.50
H	TMVW-t	MT20	4.0	6.0	2.00	1.00
I	TMVW-p	MT20	4.0	6.0	1.00	3.00
K	BMV1+p	MT20	2.0	4.0	2.25	1.00
L	BBVW-m	MT20	6.0	7.0	1.75	2.50
M	BBVW-t	MT20	6.0	7.0		
N	BMVW-t	MT20	4.0	4.0	2.00	1.75
O	BMVW-t	MT20	4.0	6.0		
P	BMVW-t	MT20	4.0	4.0	2.00	1.75
Q	BBVW-t	MT20	6.0	7.0		
R	BBVW-m	MT20	6.0	7.0	1.75	2.50
S	BMV1+p	MT20	2.0	4.0	2.25	1.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	UPLIFT	IN-SX
S	1976	0	0	5-8
K	1976	0	0	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1401	899 / 0	0 / 0	0 / 0	0 / 0	503 / 0	0 / 0
K	1401	899 / 0	0 / 0	0 / 0	0 / 0	503 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.85 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)	
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	-1753 / 0	-78.0	-78.0 0.12 (1)	4.91	R-B	-1966 / 0	0.33 (1)
B-C	-3018 / 0	-78.0	-78.0 0.17 (1)	3.85	B-Q	0 / 1687	0.42 (1)
C-D	-2461 / 0	-78.0	-78.0 0.20 (1)	4.19	Q-C	0 / 403	0.10 (1)
D-E	-2307 / 0	-78.0	-78.0 0.21 (1)	4.28	C-P	-561 / 0	0.14 (1)
E-F	-2307 / 0	-78.0	-78.0 0.21 (1)	4.28	P-D	0 / 865	0.21 (1)
F-G	-2461 / 0	-78.0	-78.0 0.20 (1)	4.19	D-O	0 / 373	0.09 (1)
G-H	-3018 / 0	-78.0	-78.0 0.17 (1)	3.85	O-E	-296 / 0	0.08 (1)
H-I	-1753 / 0	-78.0	-78.0 0.12 (1)	4.91	E-F	0 / 373	0.09 (1)
S-A	-1748 / 0	0.0	0.0 0.19 (1)	6.26	N-F	0 / 865	0.21 (1)
K-I	-1748 / 0	0.0	0.0 0.19 (1)	6.26	N-G	-561 / 0	0.14 (1)
					M-G	0 / 403	0.10 (1)
T-S	0 / 0	-96.5	-96.5 0.17 (1)	10.00	M-H	0 / 1687	0.42 (1)
S-R	0 / 0	-18.5	-18.5 0.17 (1)	10.00	L-H	-1966 / 0	0.33 (1)
R-Q	0 / 1827	-18.5	-18.5 0.33 (1)	10.00	A-R	0 / 1604	0.40 (1)
Q-U	0 / 2513	-18.5	-18.5 0.90 (1)	10.00	L-I	0 / 1604	0.40 (1)
U-P	0 / 2513	-112.0	-112.0 0.90 (1)	10.00			
P-O	0 / 2063	-112.0	-112.0 0.86 (1)	10.00			
O-N	0 / 2063	-112.0	-112.0 0.86 (1)	10.00			
N-V	0 / 2513	-112.0	-112.0 0.90 (1)	10.00			
V-M	0 / 2513	-18.5	-18.5 0.90 (1)	10.00			
M-L	0 / 1827	-18.5	-18.5 0.33 (1)	10.00			
L-K	0 / 0	-18.5	-18.5 0.17 (1)	10.00			
K-J	0 / 0	-96.5	-96.5 0.17 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
U	3-9-4	-423	-423	—	FRONT	VERT	TOTAL
V	14-8-12	-423	-423	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	34.4	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CStdGirder

START DISTANCE = 3-9-4
START SPAN CARRIED = 5-10-8
END DISTANCE = 14-8-12
END SPAN CARRIED = 5-10-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDTL. LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***

ADDTL. USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

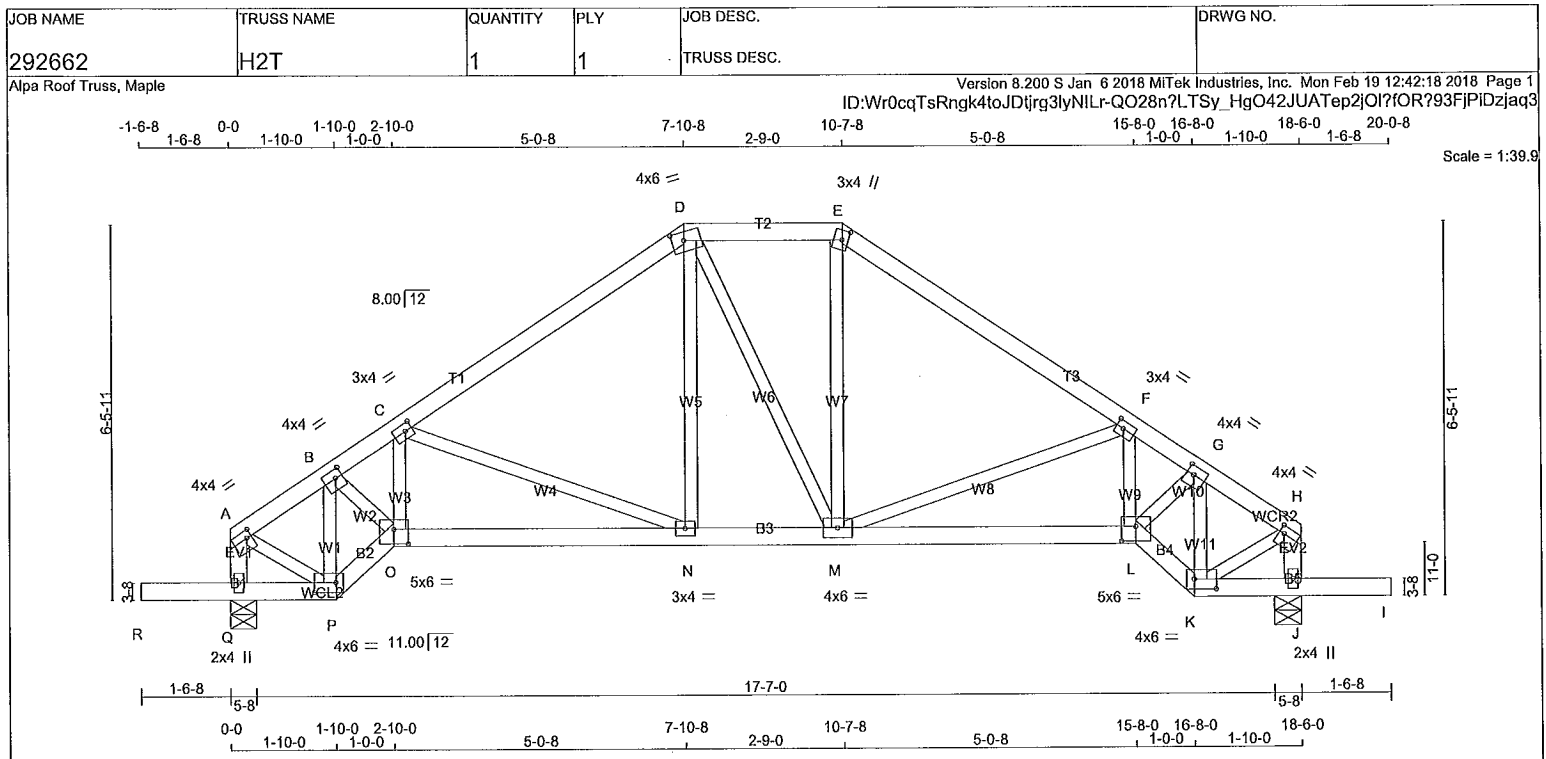
ALLOWABLE DEFL.(LL) = L/360 (0.62")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (0.62")
CALCULATED VERT. DEFL.(TL) = L/999 (0.15")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL) = L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.21/1.00 (D-E:1), BC=0.90/1.00 (P-Q:1),
WB=0.42/1.00 (H-M:1), SSI=0.30/1.00 (P-Q:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00



TOTAL WEIGHT = 84 lb
(M)(F)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
D - E	2x4	DRY No.2	SPF
E - H	2x4	DRY No.2	SPF
Q - A	2x4	DRY No.2	SPF
J - H	2x4	DRY No.2	SPF
R - P	2x4	DRY No.2	SPF
P - O	2x4	DRY No.2	SPF
O - L	2x4	DRY No.2	SPF
L - K	2x4	DRY No.2	SPF
K - I	2x4	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-I	MT20	4.0	4.0	1.50	1.00
B	TMVW-I	MT20	4.0	4.0	1.75	1.50
C	TMVW-I	MT20	3.0	4.0	1.50	1.50
D	TTWV-m	MT20	4.0	6.0	1.75	2.50
E	TTW+m	MT20	3.0	4.0	2.00	1.25
F	TMVW-I	MT20	3.0	4.0	1.50	1.50
G	TMVW-I	MT20	4.0	4.0	1.75	1.50
H	TMVW-I	MT20	4.0	4.0	1.50	1.00
J	BMV1+p	MT20	2.0	4.0		
K	BBVW-I	MT20	4.0	6.0	2.00	4.50
L	BBVW-I	MT20	5.0	6.0	3.00	3.00
M	BMVWW-I	MT20	4.0	6.0		
N	BMVW-I	MT20	3.0	4.0		
O	BBVW-I	MT20	5.0	6.0	3.00	3.00
P	BBVW-I	MT20	4.0	6.0	2.00	4.50
Q	BMV1+p	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	DOWN	IN-SX	IN-SX
Q	1041 0	1041 0	5-8	1-9
J	1041 0	1041 0	5-8	1-9

UNFACTORED REACTIONS

	1ST LCASE COMBINED	MAX./MIN. SNOW	MIN. COMPONENT LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	742	453 / 0	0 / 0	0 / 0	0 / 0	289 / 0	0 / 0
J	742	453 / 0	0 / 0	0 / 0	0 / 0	289 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.42 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LC1 (LC)	
FR-TO		FROM TO		FR-TO			
A-B	-772 / 0	-78.0 -78.0	0.07 (1)	P-B	-879 / 0	0.14 (1)	
B-C	-1319 / 0	-78.0 -78.0	0.20 (1)	B-O	0 / 839	0.19 (1)	
C-D	-921 / 0	-78.0 -78.0	0.23 (1)	O-C	0 / 55	0.02 (4)	
D-E	-755 / 0	-78.0 -78.0	0.08 (1)	C-N	-460 / 0	0.23 (1)	
E-F	-922 / 0	-78.0 -78.0	0.23 (1)	N-D	0 / 236	0.05 (1)	
F-G	-1319 / 0	-78.0 -78.0	0.20 (1)	D-M	0 / 2	0.00 (1)	
G-H	-772 / 0	-78.0 -78.0	0.07 (1)	M-E	0 / 238	0.05 (1)	
Q-A	-813 / 0	0.0 0.0	0.08 (1)	M-F	-459 / 0	0.23 (1)	
J-H	-813 / 0	0.0 0.0	0.08 (1)	L-F	-1 / 54	0.02 (4)	
				L-G	0 / 839	0.19 (1)	
R-Q	0 / 0	-96.5 -96.5	0.16 (1)	K-G	-879 / 0	0.14 (1)	
Q-P	0 / 0	-18.5 -18.5	0.12 (1)	A-P	0 / 701	0.16 (1)	
P-O	0 / 797	-18.5 -18.5	0.13 (1)	K-H	0 / 701	0.16 (1)	
O-N	0 / 1178	-18.5 -18.5	0.25 (1)				
N-M	0 / 754	-18.5 -18.5	0.17 (1)				
M-L	0 / 1179	-18.5 -18.5	0.25 (1)				
L-K	0 / 797	-18.5 -18.5	0.13 (1)				
K-J	0 / 0	-18.5 -18.5	0.12 (1)				
J-I	0 / 0	-96.5 -96.5	0.16 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 21.0 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 34.4 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.62")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.62")
CALCULATED VERT. DEFL.(TL) = L/999 (0.07")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.23/1.00 (E-F:1), BC=0.25/1.00 (N-O:1), WB=0.23/1.00 (C-N:1), SSI=0.15/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

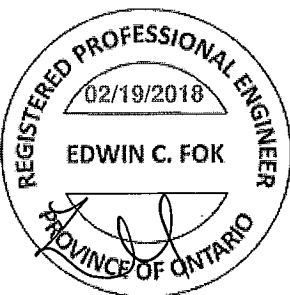
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

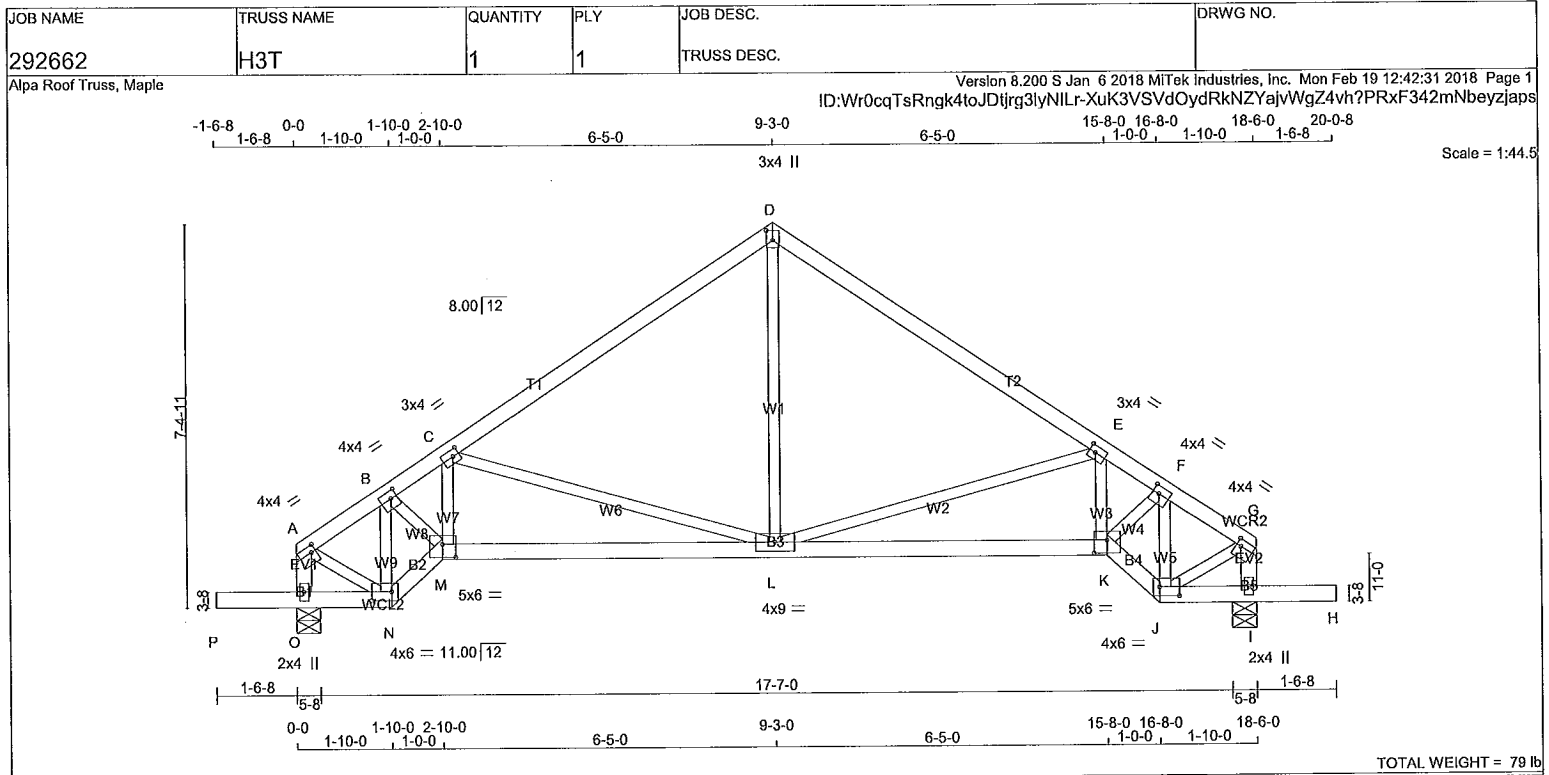
PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.30 (H) (INPUT = 1.00)

A-18023979





LUMBER				DESCR.
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
O - A	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	4.0	1.50	1.00
B	TMVW-t	MT20	4.0	4.0	1.75	1.50
C	TMVW-t	MT20	3.0	4.0	1.50	1.50
D	TTW+p	MT20	3.0	4.0	2.25	1.50
E	TMVW-t	MT20	3.0	4.0	1.50	1.50
F	TMVW-t	MT20	4.0	4.0	1.75	1.50
G	TMVW-t	MT20	4.0	4.0	1.50	1.00
I	BMV1+p	MT20	2.0	4.0		
J	BBVW-t	MT20	4.0	6.0	2.00	4.50
K	BBVW-t	MT20	5.0	6.0	3.00	3.00
L	BBVW-t	MT20	4.0	9.0		
M	BBVW-t	MT20	5.0	6.0	3.00	3.00
N	BBVW-t	MT20	4.0	6.0	2.00	4.50
O	BMV1+p	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	HORZ	BRG	UPLIFT	IN-SX	IN-SX
O	1041	0	1041	0	0	5-8	1-9	1-9	1-9
I	1041	0	1041	0	0	5-8	1-9	1-9	1-9

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	742	453 / 0	0 / 0	0 / 0	0 / 0	0 / 0	289 / 0	0 / 0
I	742	453 / 0	0 / 0	0 / 0	0 / 0	0 / 0	289 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.22 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)	MAX. CSI (LC)	UNBRAC LENGTH
FR-TO		FROM TO		FR-TO			
A-B	-765 / 0	-78.0	-78.0 0.09 (1)	L-D	0 / 495	0.11 (1)	
B-C	-1324 / 0	-78.0	-78.0 0.34 (1)	L-E	-608 / 0	0.49 (1)	
C-D	-829 / 0	-78.0	-78.0 0.38 (1)	K-E	-85 / 40	0.01 (4)	
D-E	-829 / 0	-78.0	-78.0 0.38 (1)	K-F	0 / 947	0.21 (1)	
E-F	-1324 / 0	-78.0	-78.0 0.34 (1)	J-F	-862 / 0	0.13 (1)	
F-G	-765 / 0	-78.0	-78.0 0.09 (1)	C-L	-608 / 0	0.49 (1)	
O-A	-813 / 0	0.0	0.0 0.08 (1)	M-C	-85 / 40	0.01 (4)	
I-G	-813 / 0	0.0	0.0 0.08 (1)	B-M	0 / 947	0.21 (1)	
P-O	0 / 0	-96.5	-96.5 0.16 (1)	N-B	-862 / 0	0.13 (1)	
O-N	0 / 0	-18.5	-18.5 0.12 (1)	A-N	0 / 687	0.15 (1)	
N-M	0 / 781	-18.5	-18.5 0.13 (1)	J-G	0 / 687	0.15 (1)	
M-L	0 / 1248	-18.5	-18.5 0.33 (4)				
L-K	0 / 1248	-18.5	-18.5 0.33 (4)				
K-J	0 / 781	-18.5	-18.5 0.13 (1)				
J-I	0 / 0	-18.5	-18.5 0.12 (1)				
I-H	0 / 0	-96.5	-96.5 0.16 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 21.0	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 34.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.62")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.62")
CALCULATED VERT. DEFL.(TL) = L/999 (0.09")

CANTILEVER DEFLECTION:

ALLOWABLE DEFL.(LL) = L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.38/1.00 (C-D:1), BC=0.33/1.00 (K-L:4), WB=0.49/1.00 (C-L:1), SS=0.23/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

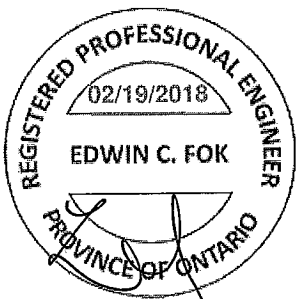
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
618	354	1667	788
1987	1656		

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (A) (INPUT = 0.90)
JSI METAL= 0.30 (G) (INPUT = 1.00)

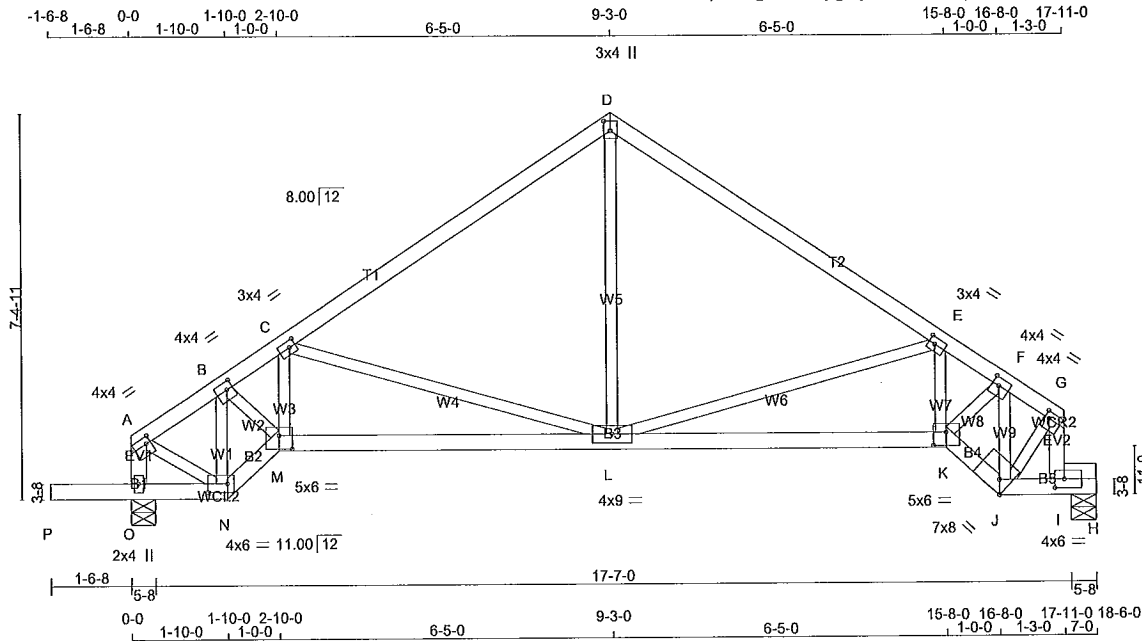
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292662	H3TS	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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Scale = 1:44.3

TOTAL WEIGHT = 77 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - G	2x4	DRY	No.2	SPF	
O - A	2x4	DRY	No.2	SPF	
I - G	2x4	DRY	No.2	SPF	
P - N	2x4	DRY	No.2	SPF	
N - M	2x4	DRY	No.2	SPF	
M - K	2x4	DRY	No.2	SPF	
K - J	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
I - H	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	4.0	1.50	1.00
B	TMWW-t	MT20	4.0	4.0	1.75	1.50
C	TMWW-t	MT20	3.0	4.0	1.50	1.50
D	TTW+p	MT20	3.0	4.0	2.25	1.50
E	TMWW-t	MT20	3.0	4.0	1.50	1.50
F	TMWW-t	MT20	4.0	4.0	1.75	1.50
G	TMVV-t	MT20	4.0	4.0	1.50	1.00
I	BMVV-t	MT20	4.0	6.0	2.00	2.25
J	BBWW-h	MT20	7.0	8.0	2.50	Edge
K	BBWW-l	MT20	5.0	6.0	3.00	3.00
L	BMWWW-t	MT20	4.0	9.0		
M	BBWW-l	MT20	5.0	6.0	3.00	3.00
N	BBWW-l	MT20	4.0	6.0	2.00	4.50
O	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	DOWN	HORZ	UPLIFT
H	841	0	841	0
O	1047	0	1047	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX /MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
H	COMBINED	362 / 0	0 / 0	0 / 0	0 / 0	239 / 0	0 / 0	0 / 0
O	746	455 / 0	0 / 0	0 / 0	0 / 0	291 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.92 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM	TO		FR-TO			
A-B	-771 / 0	-78.0	-78.0	0.10 (1)	6.25	N-B	-868 / 0	0.13 (1)
B-C	-1334 / 0	-78.0	-78.0	0.44 (1)	4.97	B-M	0 / 953	0.21 (1)
C-D	-839 / 0	-78.0	-78.0	0.48 (1)	5.96	M-C	-85 / 40	0.01 (4)
D-E	-839 / 0	-78.0	-78.0	0.48 (1)	5.95	C-L	-608 / 0	0.49 (1)
E-F	-1361 / 0	-78.0	-78.0	0.44 (1)	4.92	L-D	0 / 506	0.11 (1)
F-G	-820 / 0	-78.0	-78.0	0.14 (1)	6.25	L-E	-647 / 0	0.53 (1)
O-A	-818 / 0	0.0	0.0	0.08 (1)	7.81	K-E	-82 / 46	0.02 (4)
I-G	-1209 / 0	0.0	0.0	0.13 (1)	7.28	K-F	0 / 976	0.22 (1)
						J-F	-792 / 0	0.12 (1)
P-O	0 / 0	-96.5	-96.5	0.16 (1)	10.00	A-N	0 / 693	0.16 (1)
O-N	0 / 0	-18.5	-18.5	0.16 (1)	10.00	J-G	0 / 914	0.21 (1)
N-M	0 / 787	-18.5	-18.5	0.13 (1)	10.00			
M-L	0 / 1256	-18.5	-18.5	0.33 (4)	10.00			
L-K	0 / 1293	-18.5	-18.5	0.34 (4)	10.00			
K-J	0 / 816	-18.5	-18.5	0.13 (1)	10.00			
J-I	0 / 8	-18.5	-18.5	0.66 (1)	10.00			
I-H	0 / 0	-18.5	-18.5	0.28 (1)	10.00			

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 21.0	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 34.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)=	L/360 (0.62")
CALCULATED VERT. DEFL.(LL)=	L/999 (0.04")
ALLOWABLE DEFL.(TL)=	L/360 (0.62")
CALCULATED VERT. DEFL.(TL)=	L/999 (0.11")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL)= L/999 (0.00")

CSI: TC=0.48/1.00 (D-E:1), BC=0.66/1.00 (I-J:1), WB=0.53/1.00 (E-L:1), SS=0.33/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

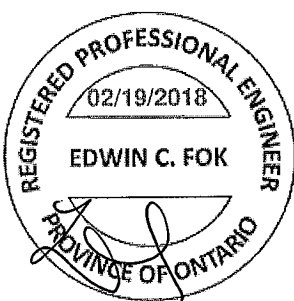
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
618	354	1667	788
1987	1656		

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (I) (INPUT = 0.90)
JSI METAL= 0.52 (I) (INPUT = 1.00)

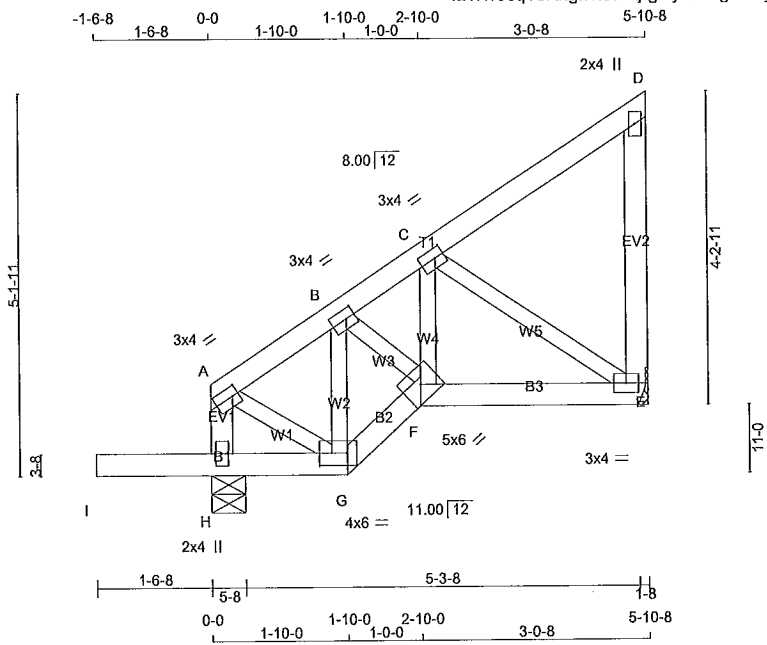
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292662	J1T	3	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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Scale = 1:31.0

TOTAL WEIGHT = 3 X 30 = 90 lb

LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	SPF	
H - A	2x4	DRY	No.2	SPF	
A - D	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	
I - G	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
F - E	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW-1	MT20	3.0	4.0	1.50 1.00
B	TMVW-1	MT20	3.0	4.0	1.50 1.50
C	TMVW-1	MT20	3.0	4.0	1.50 1.50
D	TMV+p	MT20	2.0	4.0	
E	BMVW1-t	MT20	3.0	4.0	
F	BBVW-h	MT20	5.0	6.0	
G	BBVW-1	MT20	4.0	6.0	2.00 4.50
H	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT 8RG	REQRD BRG
JT		VERT	HORZ	DOWN	HORZ
H	452	0	452	0	0
E	264	0	264	0	0

UNFACTORED REACTIONS		1ST LCASE	MAX/MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
JT		COMBINED	SNOW	LIVE	PERM.LIVE	
H	322	197 / 0	0 / 0	0 / 0	0 / 0	125 / 0
E	188	115 / 0	0 / 0	0 / 0	0 / 0	73 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

CHORDS		MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. FACTORED (LC)	MAX. FACTORED UNBRACED LENGTH	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED (LC)
MEMB.						MEMB.		
FR-TO						FR-TO		
H-A	-223 / 0	0.0	0.0	0.02 (1)	7.81	A-G	0 / 154	0.03 (1)
A-B	-165 / 0	-78.0	-78.0	0.04 (1)	6.25	G-B	-220 / 0	0.03 (1)
B-C	-239 / 0	-78.0	-78.0	0.08 (1)	6.25	B-F	0 / 130	0.03 (1)
C-D	-11 / 0	-78.0	-78.0	0.08 (1)	6.25	F-C	0 / 63	0.02 (4)
E-D	-95 / 0	0.0	0.0	0.03 (1)	7.81	C-E	-259 / 0	0.06 (1)
I-H	0 / 0	-96.5	-96.5	0.16 (1)	10.00			
H-G	0 / 0	-18.5	-18.5	0.16 (1)	10.00			
G-F	0 / 173	-18.5	-18.5	0.03 (1)	10.00			
F-E	0 / 216	-18.5	-18.5	0.07 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/588 (0.03")

CSI: TC=0.08/1.00 (B-C:1), BC=0.16/1.00 (G-H:1), WB=0.06/1.00 (C-E:1), SSI=0.12/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

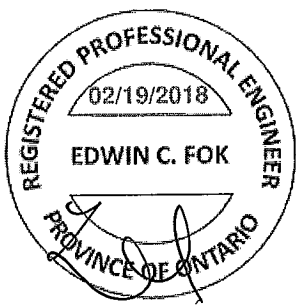
NAIL VALUES		PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
		(PSI)	(PLI)	(PLI)
		MAX MIN	MAX MIN	MAX MIN
MT20		618 354	1667 788	1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

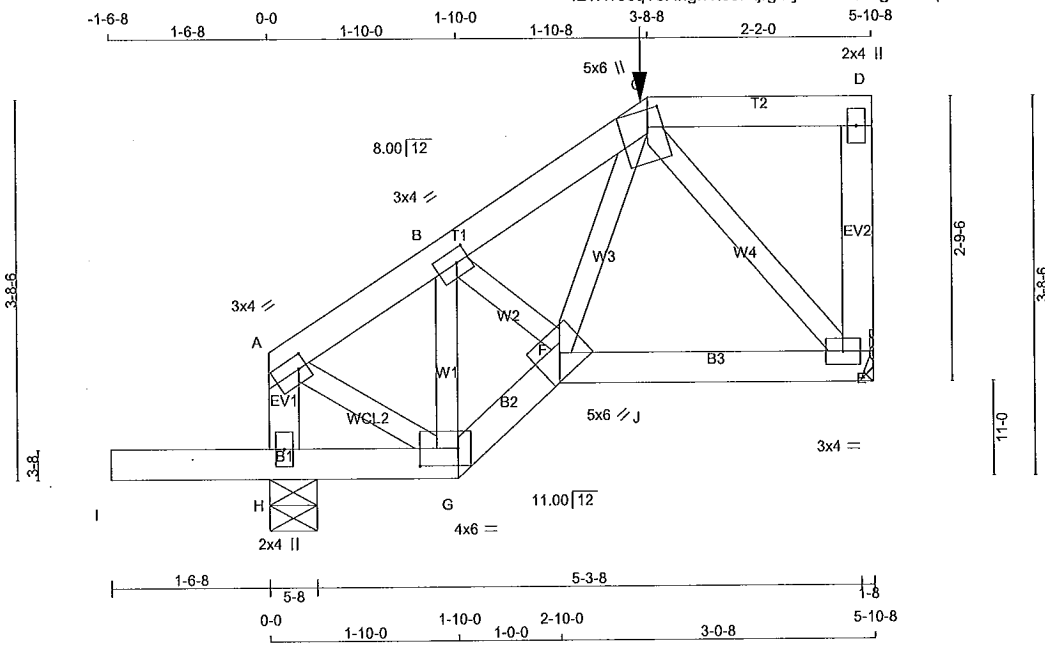
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.50 (A) (INPUT = 0.90)
JSI METAL= 0.08 (E) (INPUT = 1.00)

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



A-18023982



Scale = 1:22.4

TOTAL WEIGHT = 2 X 28 = 57 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	3.0	4.0	1.50	1.00
B	TMVW-t	MT20	3.0	4.0	1.50	1.50
C	TTVW+m	MT20	5.0	6.0	2.00	1.75
D	TMV+p	MT20	2.0	4.0		
E	BMVW-t	MT20	3.0	4.0		
F	BBVW-h	MT20	5.0	6.0		
G	BBVW-l	MT20	4.0	6.0	2.00	4.50
H	BMV1+p	MT20	2.0	4.0		

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 106.4 lbs FACTORED DOWN AT 3-8-8 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
E	413	0	413	0	0	0
H	527	0	527	0	0	0

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
E	294	183 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0
H	377	225 / 0	0 / 0	0 / 0	0 / 0	152 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED		FACTORED	
MEMB.	FORCE	VERT. LOAD	LC1 MAX	MEMB.	FORCE	MAX	
	(LBS)	(PLF)	CSI (LC)	UNBRAC	(LBS)	CSI (LC)	
FR-TO		FROM TO		LENGTH	FR-TO		
A-B	-243 / 0	-78.0 -78.0	0.04 (1)	6.25	G-B	-302 / 0	0.05 (1)
B-C	-339 / 0	-78.0 -78.0	0.04 (1)	6.25	B-F	0 / 118	0.03 (1)
C-D	0 / 0	-111.3 -111.3	0.10 (1)	10.00	F-C	0 / 157	0.04 (1)
E-D	-121 / 0	0.0 0.0	0.02 (1)	7.81	C-E	-338 / 0	0.07 (1)
H-A	-292 / 0	0.0 0.0	0.03 (1)	7.81	A-G	0 / 231	0.06 (1)
I-H	0 / 0	-96.5 -96.5	0.17 (1)	10.00			
H-G	0 / 0	-26.4 -26.4	0.17 (1)	10.00			
G-F	0 / 261	-26.4 -26.4	0.05 (1)	10.00			
F-J	0 / 223	-26.4 -26.4	0.10 (4)	10.00			
J-E	0 / 223	-26.4 -26.4	0.10 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	3-8-8	-106	-106	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHlp
LEFT SETBACK = 3-8-8
RIGHT SETBACK = 0-0
END SETBACK = 3-8-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADD'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL)= L/999 (0.01")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL)= L/617 (0.03")

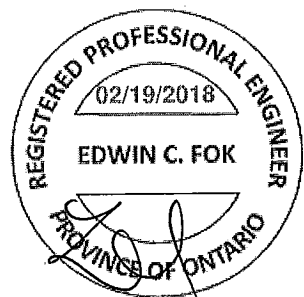
CSI: TC=0.10/1.00 (C-D:1), BC=0.17/1.00 (G-H:1), WB=0.07/1.00 (C-E:1), SSI=0.13/1.00 (I-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

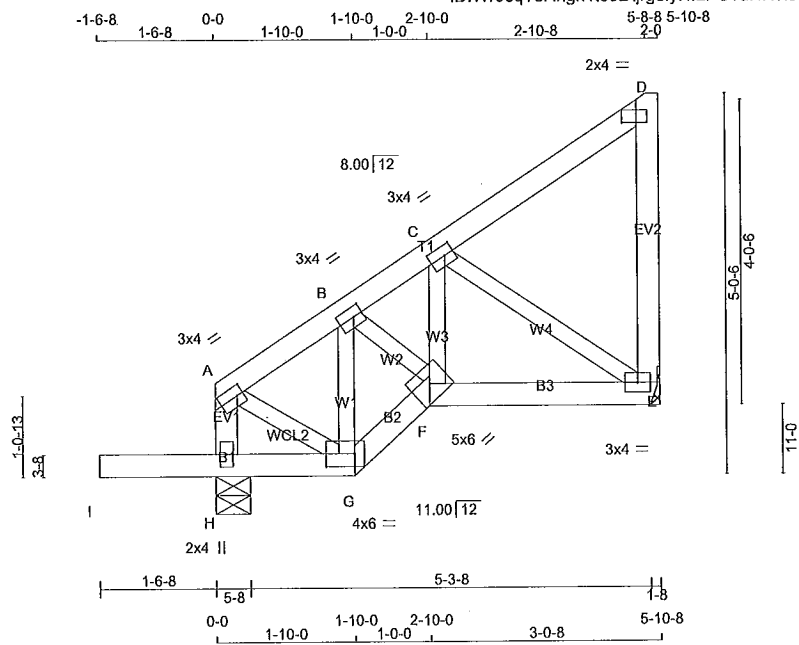
NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292662	J1TB	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Feb 19 12:44:16 2018 Page 1
ID:Wr0cqTsRngk4toJDtjrg3lyNlLr-UvdNK?lsuvjhCMWxfYnvuUOIawxXJOgOyyZ6JxzjaoE



Scale = 1:30.5

TOTAL WEIGHT = 2 X 30 = 60 lb

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW-t	MT20	3.0	4.0	1.50 1.00
B	TMVW-t	MT20	3.0	4.0	1.50 1.50
C	TMVW-t	MT20	3.0	4.0	1.50 1.50
D	TVM-p	MT20	2.0	4.0	0.50 2.25
E	BMVW1-t	MT20	3.0	4.0	
F	BBWW-h	MT20	5.0	6.0	
G	BBWW-l	MT20	4.0	6.0	2.00 4.50
H	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT	REQRD
JT	GROSS REACTION	GROSS REACTION	GROSS REACTION	DOWN	UP	BRG	BRG
E	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
	264	0	264	0	0	HANGER BY OTHERS	
H	452	0	452	0	0	5-8	1-8

UNFACTORED REACTIONS							
	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	188	115 / 0	0 / 0	0 / 0	0 / 0	73 / 0	0 / 0
H	322	197 / 0	0 / 0	0 / 0	0 / 0	125 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MAX. FACTORED		FACTORED			MAX. FACTORED				
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRAC	MEMB.	FORCE (LBS)	MAX		
FR-TO		FROM TO	CSI (LC)	LENGTH	FR-TO		CSI (LC)		
A-B	-165 / 0	-78.0	-78.0	0.04 (1)	6.25	G-B	-220 / 0	0.03 (1)	
B-C	-239 / 0	-78.0	-78.0	0.08 (1)	6.25	B-F	0 / 130	0.03 (1)	
C-D	-11 / 0	-78.0	-78.0	0.08 (1)	6.25	F-C	0 / 63	0.02 (4)	
E-D	-95 / 0	0.0	0.0	0.03 (1)	7.81	C-E	-259 / 0	0.06 (1)	
H-A	-223 / 0	0.0	0.0	0.02 (1)	7.81	A-G	0 / 154	0.03 (1)	
I-H	0 / 0	-96.5	-96.5	0.16 (1)	10.00				
H-G	0 / 0	-18.5	-18.5	0.16 (1)	10.00				
G-F	0 / 173	-18.5	-18.5	0.03 (1)	10.00				
F-E	0 / 216	-18.5	-18.5	0.07 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL)= L/999 (0.01")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL)= L/588 (0.03")

CSI: TC=0.08/1.00 (B-C:1), BC=0.16/1.00 (G-H:1), WB=0.06/1.00 (C-E:1), SSI=0.12/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 L S BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

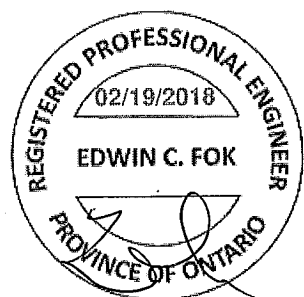
NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PSI)	(PLI)
MT20	618	354	1667
	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

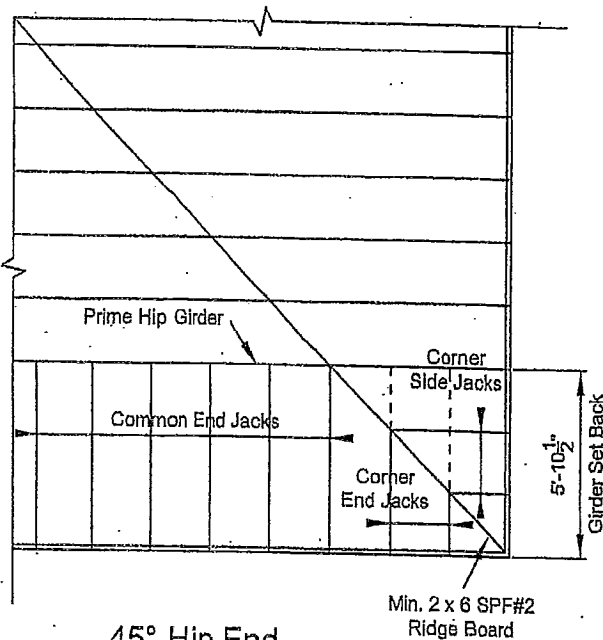
JSI GRIP= 0.50 (A) (INPUT = 0.90)
JSI METAL= 0.08 (D) (INPUT = 1.00)

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



A-18023984

STRACON ENGINEERING INC.



45° Hip End

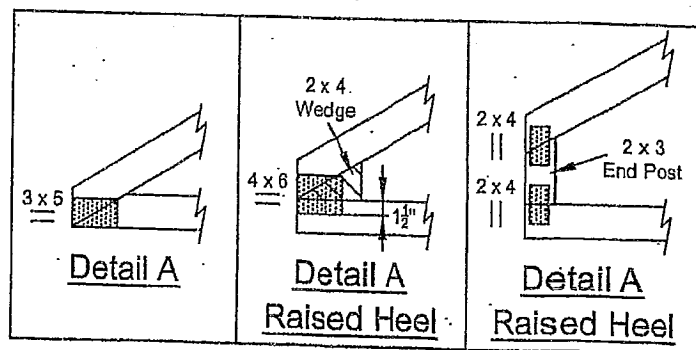
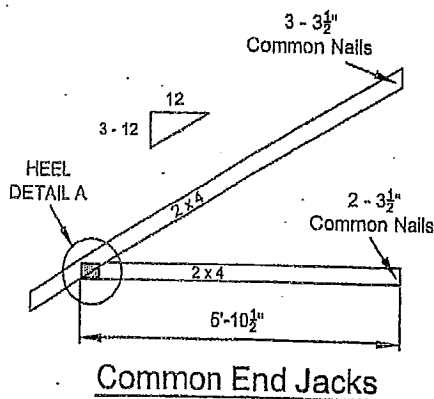
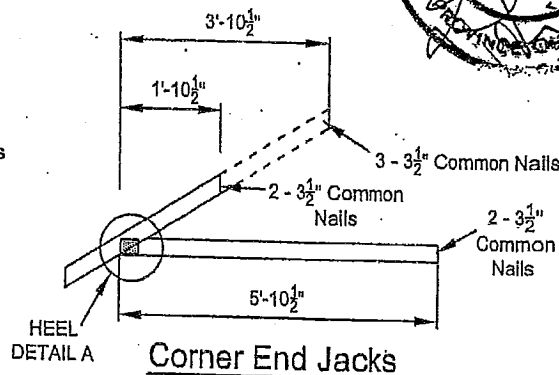
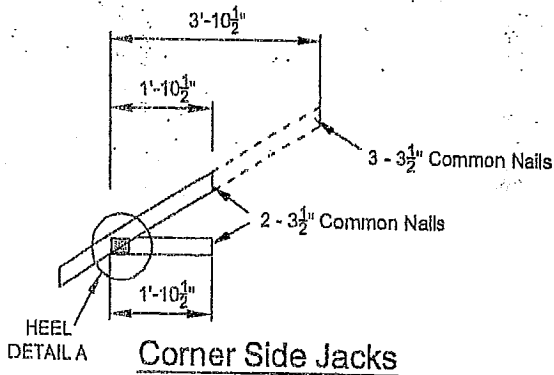
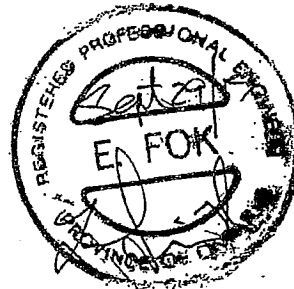
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD : 50.5 P.S.F



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

CS-51008

HHUS – Double Shear Joist Hangers

All HHUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 14 gauge

Finish: G90 galvanized

Design:

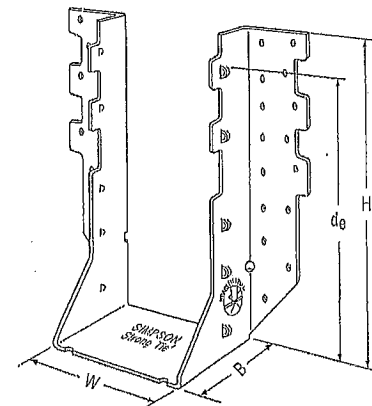
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

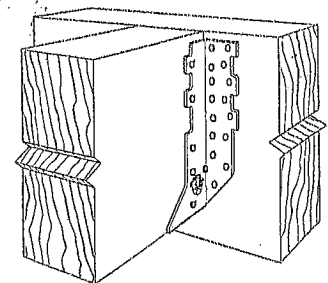
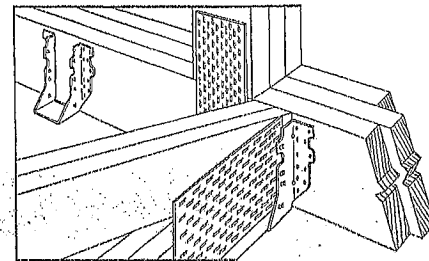
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

- See current catalogue for options

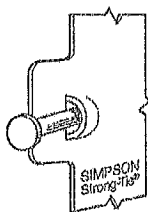


HHUS410

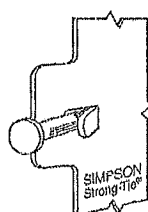


Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _g ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _o =1.15)	Normal (K _o =1.00)	Uplift (K _o =1.15)	Normal (K _o =1.00)
HHUS26-2	14	3 1/16	5 13/16	3	3 15/16	(14) 16d	(6) 16d	2850	7335	2065	5205
HHUS28-2	14	3 5/16	7 1/2	3	6 5/32	(22) 16d	(8) 16d	3765	8940	2675	6345
HHUS210-2	14	3 1/16	9 5/32	3	8	(30) 16d	(10) 16d	4745	9660	4310	7000
HHUS210-3	14	4 1/16	9	3	7 15/16	(30) 16d	(10) 16d	4745	10545	4310	7485
HHUS210-4	14	5 1/16	8 29/32	3	7 27/32	(30) 16d	(10) 16d	4745	10545	4310	7485
HHUS46	14	3 3/8	5 13/32	3	3 15/16	(14) 16d	(6) 16d	2540	7335	2065	5205
HHUS48	14	3 3/8	7 1/8	3	6 1/8	(22) 16d	(8) 16d	3765	8945	2267	6345
HHUS410	14	3 5/8	9	3	8	(30) 16d	(10) 16d	4745	9855	4310	7000
HHUS5.50/10	14	5 1/2	9	3	8	(30) 16d	(10) 16d	4745	10545	4310	7485
HHUS7.25/10	14	7 1/4	9	3 3/16	7 29/32	(30) 16d	(10) 16d	4745	10770	4310	7650

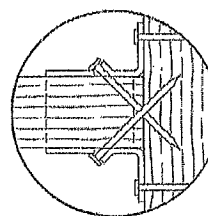
1. d_g is the distance from the seat of the hanger to the highest joist nail.



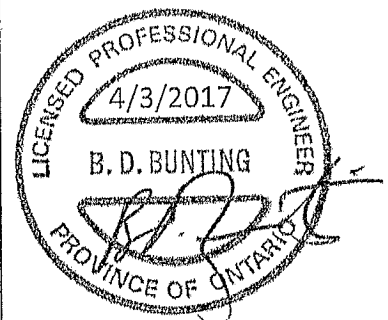
U.S. Patent
5,603,580



Double Shear Nailing Side View.
Do not bend tab back.



Double Shear Nailing Top View.



LIMIT
STATES
DESIGN

This technical bulletin is effective until June 30, 2019, and reflects information available as of March 1, 2017. This information is updated periodically and should not be relied upon after June 30, 2019. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECHHUS17-3/17 exp. 6/19

(800) 999-5099
strongtie.com

LUS – Double Shear Joist Hangers

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

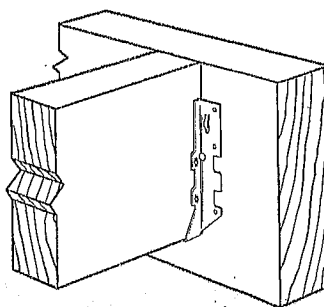
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

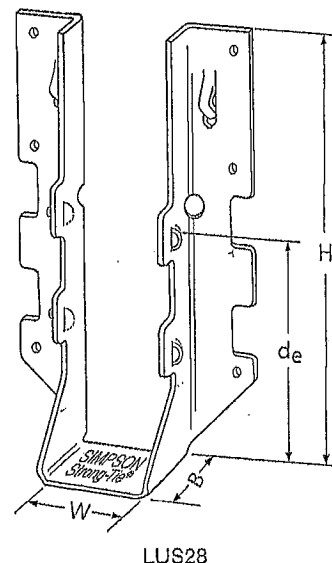
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified



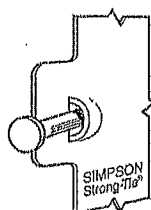
Typical LUS
Installation



LUS28

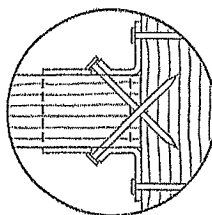
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _D =1.15)	Normal (K _D =1.00)	Uplift (K _D =1.15)	Normal (K _D =1.00)
LUS24	18	1½	3½	1½	1½	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3½	3½	2	1½	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1½	4¾	1¾	3¾	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3½	4¾	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4¾	4¾	2	3¼	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1½	6¾	1¾	3¾	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3½	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4¾	6¼	2	3¼	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1½	7½	1¾	3¾	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3½	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4¾	8¾	2	5¼	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.

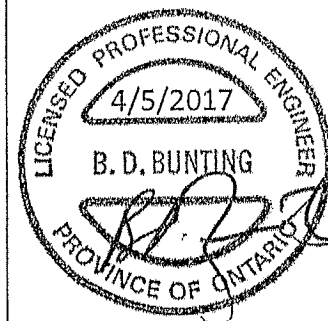


Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).

U.S. Patent
5,603,580



Double
Shear
Nailing
Top View.



LIMIT
STATES
DESIGN

This technical bulletin is effective until June 30, 2019, and reflects information available as of March 1, 2017. This information is updated periodically and should not be relied upon after June 30, 2019. Contact Simpson Strong-Tie for current information and limited warranty of products on file.

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