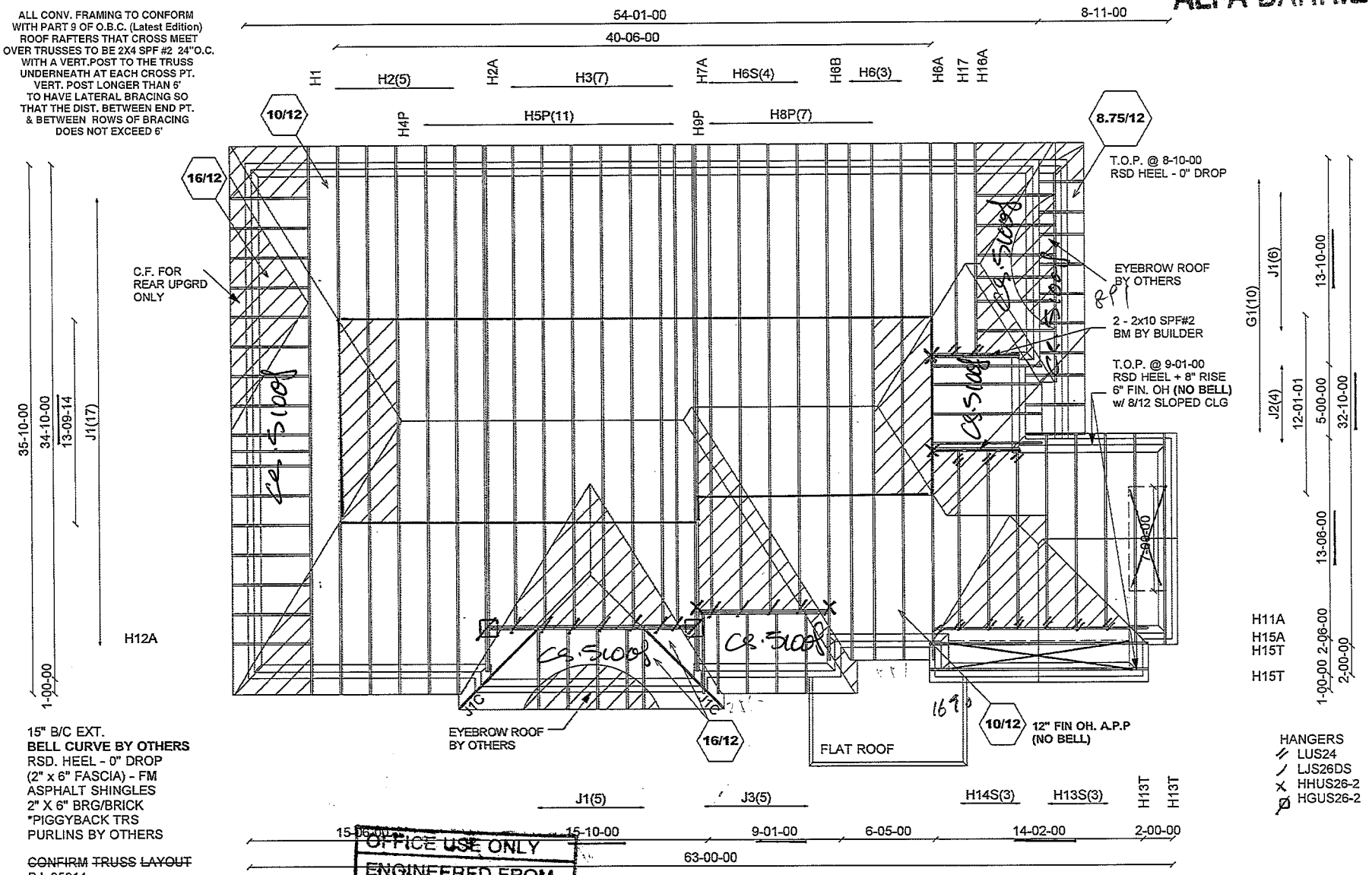
ALPA-BARRIE

ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. (Latest Edition) ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2X4 SPF #2 24"O.C. WITH A VERT.POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DIST. BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'



15" B/C EXT.
BELL CURVE BY OTHERS
RSD. HEEL - 0" DROP
(2" x 6" FASCIA) - FM
ASPHALT SHINGLES
2" X 6" BRG/BRICK
*PIGGYBACK TRS
PURLINS BY OTHERS

~~CONFIRM TRUSS LAYOUT~~
P.L.95914

HANGERS
// LUS24
/ LJS26DS
X HHUS26-2
Ø HGUS26-2

CONVENTIONAL FRAMING BY OTHERS

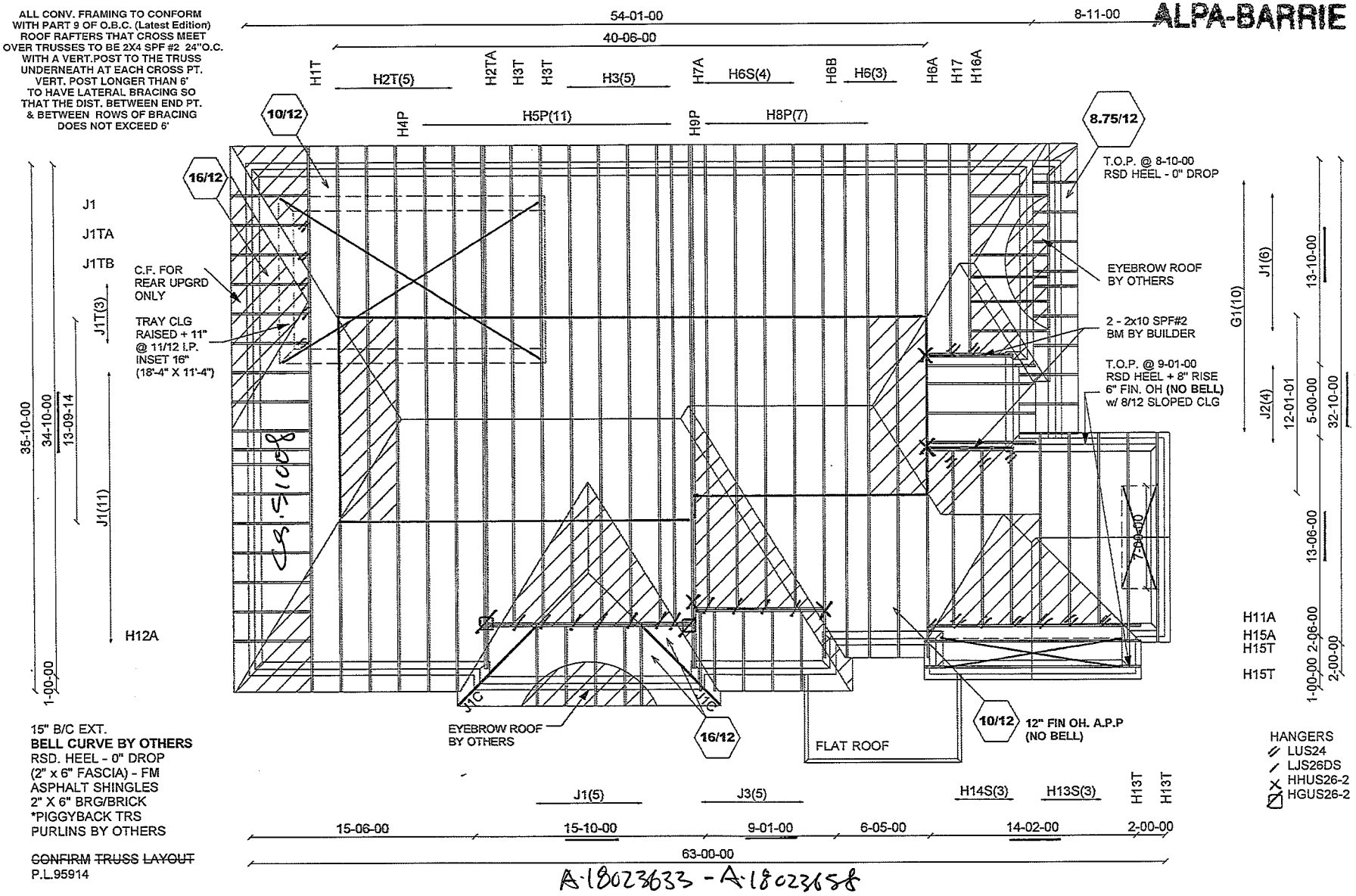
OFFICE USE ONLY
ENGINEERED FROM
A- 18023630 TO
A- 18023651

BROOKVALLEY

Job Track: 45147	Builder / Location: GOLD PARK HOMES / VAUGHAN	Model / Elevation: 4204 REV1	/ A STD OR OPT 2ND FLR (SBED) OR REAR UPG.
Layout ID: 292613			
Plan Log: 95914	Project: PINE VALLEY		
	Date: 1/18/2018 Designer: AMANDA		

ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. (Latest Edition) ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2X4 SPF #2 24"O.C. WITH A VERT.POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DIST. BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

ALPA-BARRIE



15" B/C EXT.
BELL CURVE BY OTHERS
RSD. HEEL - 0" DROP
(2" x 6" FASCIA) - FM
ASPHALT SHINGLES
2" X 6" BRG/BRICK
*PIGGYBACK TRS
PURLINS BY OTHERS

~~CONFIRM TRUSS LAYOUT~~
P.L.95914

HANGERS
LUS24
LJS26DS
HHUS26-2
HGUS26-2

A-18023633 - A-18023658

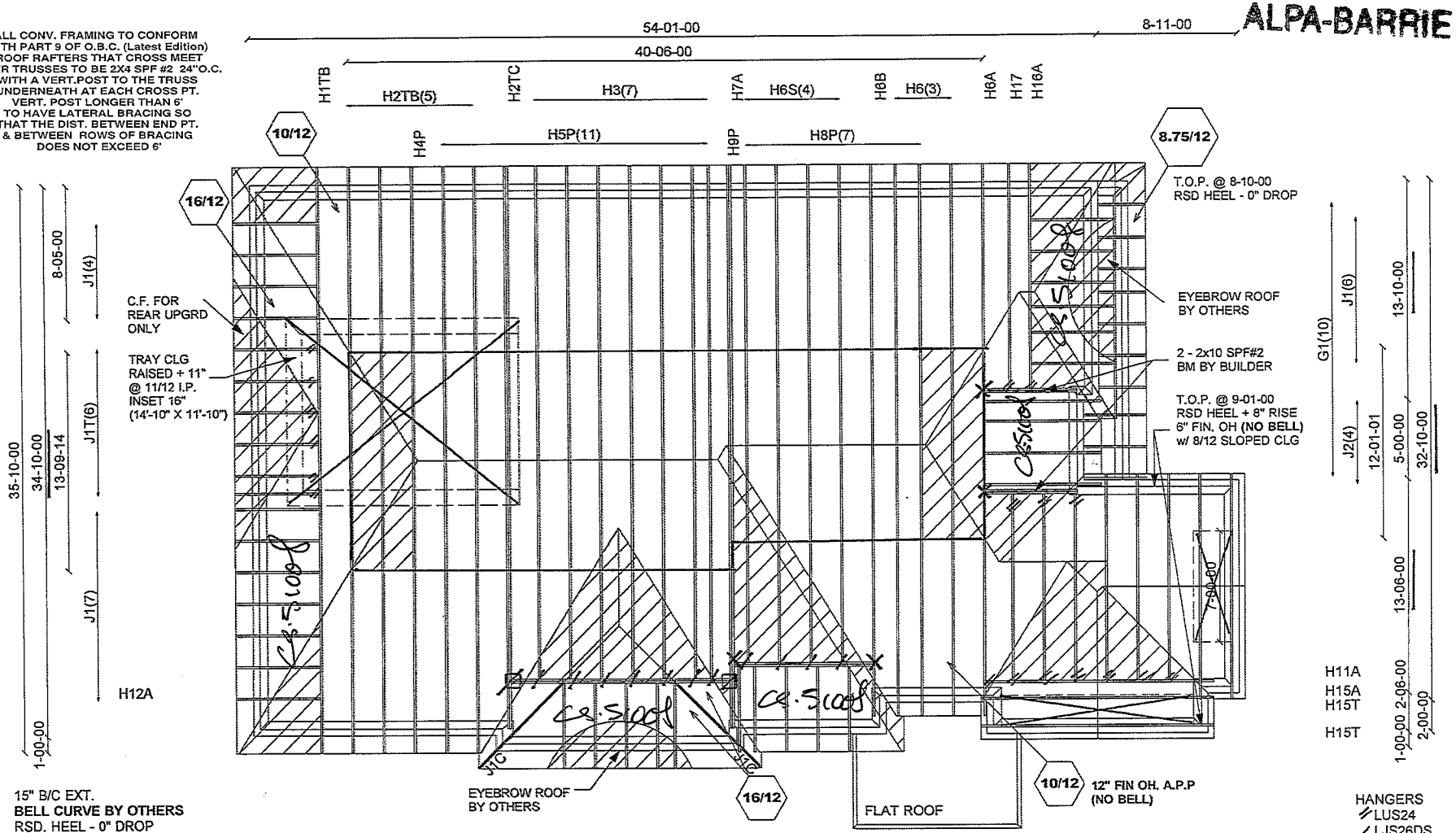
CONVENTIONAL FRAMING BY OTHERS

BROOKVALLEY

Job Track: 45147	Builder / Location: GOLD PARK HOMES / VAUGHAN	Model / Elevation: 4204 REV1	OR REAR UPG. / A OPT TRAY
Layout ID: 292614	Project: PINE VALLEY		
Plan Log: 95914	Date: 1/18/2018 Designer: AMANDA		

ALPA-BARRIE

ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. (Latest Edition) ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2X4 SPF #2 24"O.C. WITH A VERT.POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DIST. BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'



15" B/C EXT.
BELL CURVE BY OTHERS
RSD. HEEL - 0" DROP
(2" x 6" FASCIA) - FM
ASPHALT SHINGLES
2" X 6" BRG/BRICK
*PIGGYBACK TRS
PURLINS BY OTHERS

~~CONFIRM TRUSS LAYOUT~~
P.L.95914

HANGERS
// LUS24
/ LJS26DS
X HHUS26-2
Ø HGUS26-2

CONVENTIONAL FRAMING BY OTHERS

A-18023633 - A-18023666
A-18023656

BROOKVALLEY

OR REAR UPG.

Job Track: 45147	Builder / Location: GOLD PARK HOMES / VAUGHAN	Model / Elevation: 4204 REV1 / A OPT TRAY 5 BED
Layout ID: 292615	Project: PINE VALLEY	
Plan Log: 95914	Date: 1/18/2018 Designer: AMANDA	

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292613	H1	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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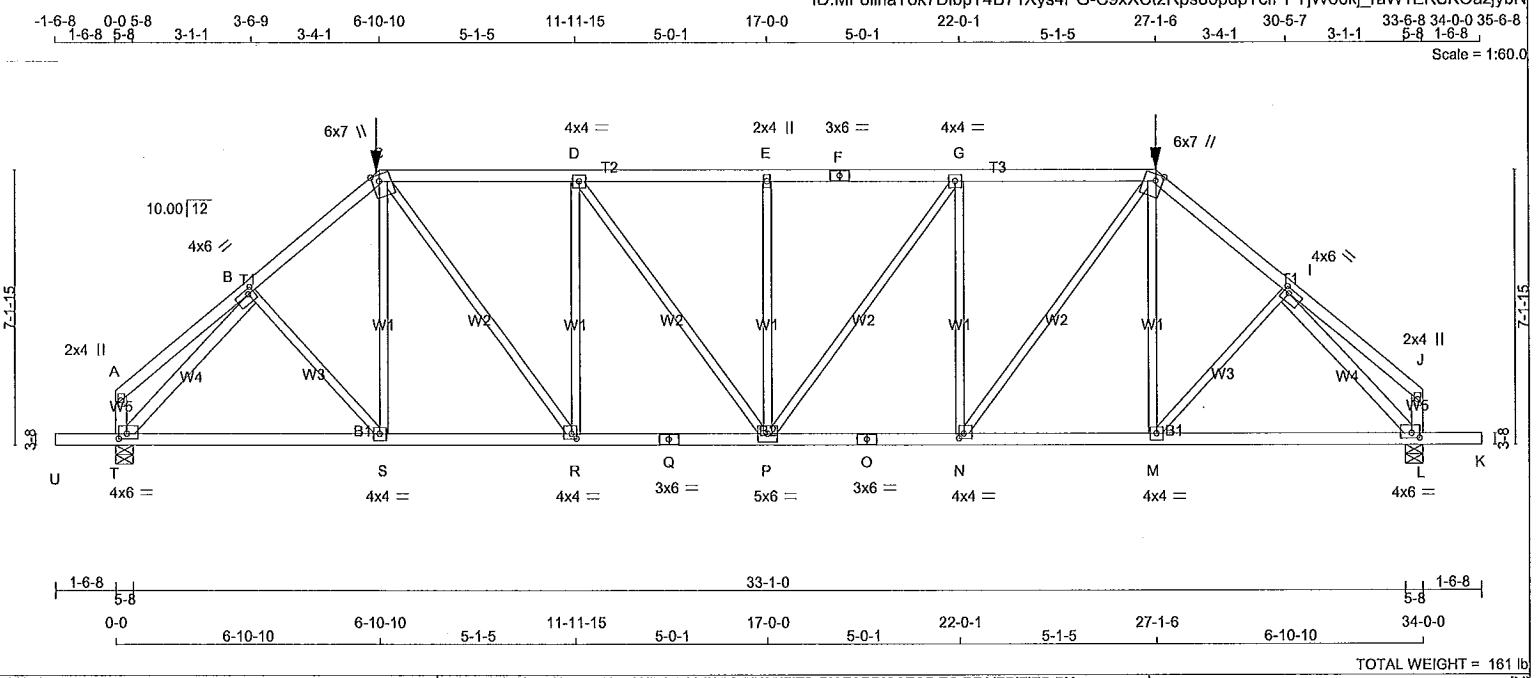


TOTAL WEIGHT = 161 lb

(M)

LUMBER

N. L. G. A. RULES

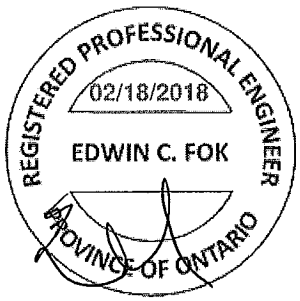
CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
T - A	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
U - Q	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0		
B	TMWW-t	MT20	4.0	6.0	1.50	1.75
C	TTWW+m	MT20	6.0	7.0	2.00	2.25
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW+m	MT20	6.0	7.0	2.00	2.25
I	TMWW-t	MT20	4.0	6.0	1.50	1.75
J	TMV+p	MT20	2.0	4.0		
L	BMVW1-t	MT20	4.0	6.0	1.50	2.50
M	BMWW-t	MT20	4.0	4.0		
N	BMWW-t	MT20	4.0	4.0	1.50	1.50
O	BS-t	MT20	3.0	6.0		
P	BMWWW-t	MT20	5.0	6.0		
Q	BS-t	MT20	3.0	6.0		
R	BMWW-t	MT20	4.0	4.0	1.50	1.50
S	BMWW-t	MT20	4.0	4.0		
T	BMVW1-t	MT20	4.0	6.0	1.50	2.50

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	2518	0	2518	0	0	5-8	2-11
L	2518	0	2518	0	0	5-8	2-11

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1796	1092 / 0	0 / 0	0 / 0	0 / 0	704 / 0	0 / 0
L	1796	1092 / 0	0 / 0	0 / 0	0 / 0	704 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.19 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX. CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 17	-78.0	-78.0	0.14 (1)	10.00	B- S	0 / 208	0.05 (1)
B- C	-2676 / 0	-78.0	-78.0	0.26 (1)	3.99	S- C	0 / 133	0.05 (4)
C- D	-2858 / 0	-114.5	-114.5	0.68 (1)	3.35	C- R	0 / 1372	0.34 (1)
D- E	-3101 / 0	-114.5	-114.5	0.70 (1)	3.19	R- D	-983 / 0	0.89 (1)
E- F	-3101 / 0	-114.5	-114.5	0.70 (1)	3.19	D- P	0 / 413	0.10 (1)
F- G	-3101 / 0	-114.5	-114.5	0.70 (1)	3.19	P- E	-527 / 0	0.48 (1)
G- H	-2858 / 0	-114.5	-114.5	0.68 (1)	3.35	P- G	0 / 413	0.10 (1)
H- I	-2676 / 0	-78.0	-78.0	0.26 (1)	3.99	N- G	-983 / 0	0.89 (1)
I- J	0 / 17	-78.0	-78.0	0.14 (1)	10.00	N- H	0 / 1372	0.34 (1)
T- A	-107 / 0	0.0	0.0	0.01 (1)	7.81	M- H	0 / 133	0.05 (4)
L- J	-107 / 0	0.0	0.0	0.01 (1)	7.81	M- I	0 / 208	0.05 (1)
						T- B	-2893 / 0	0.93 (1)
U- T	0 / 0	-96.5	-96.5	0.17 (1)	10.00	I- L	-2893 / 0	0.93 (1)
T- S	0 / 1897	-27.2	-27.2	0.51 (1)	10.00			
S- R	0 / 2038	-27.2	-27.2	0.54 (1)	10.00			
R- Q	0 / 2858	-27.2	-27.2	0.57 (1)	10.00			
Q- P	0 / 2858	-27.2	-27.2	0.57 (1)	10.00			
P- O	0 / 2858	-27.2	-27.2	0.57 (1)	10.00			
O- N	0 / 2858	-27.2	-27.2	0.57 (1)	10.00			
N- M	0 / 2038	-27.2	-27.2	0.54 (1)	10.00			
M- L	0 / 1897	-27.2	-27.2	0.51 (1)	10.00			
L- K	0 / 0	-96.5	-96.5	0.17 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	6-10-10	-213	-213	---	FRONT	VERT	TOTAL
H	27-1-6	-213	-213	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 6-10-10
END SETBACK = 3-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.13")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (1.13")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.26")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.70/1.00 (D-E:1) , BC=0.57/1.00 (P-R:1) , WB=0.93/1.00 (I-L:1) , SSI=0.30/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

A-18023630

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292613	H2	5	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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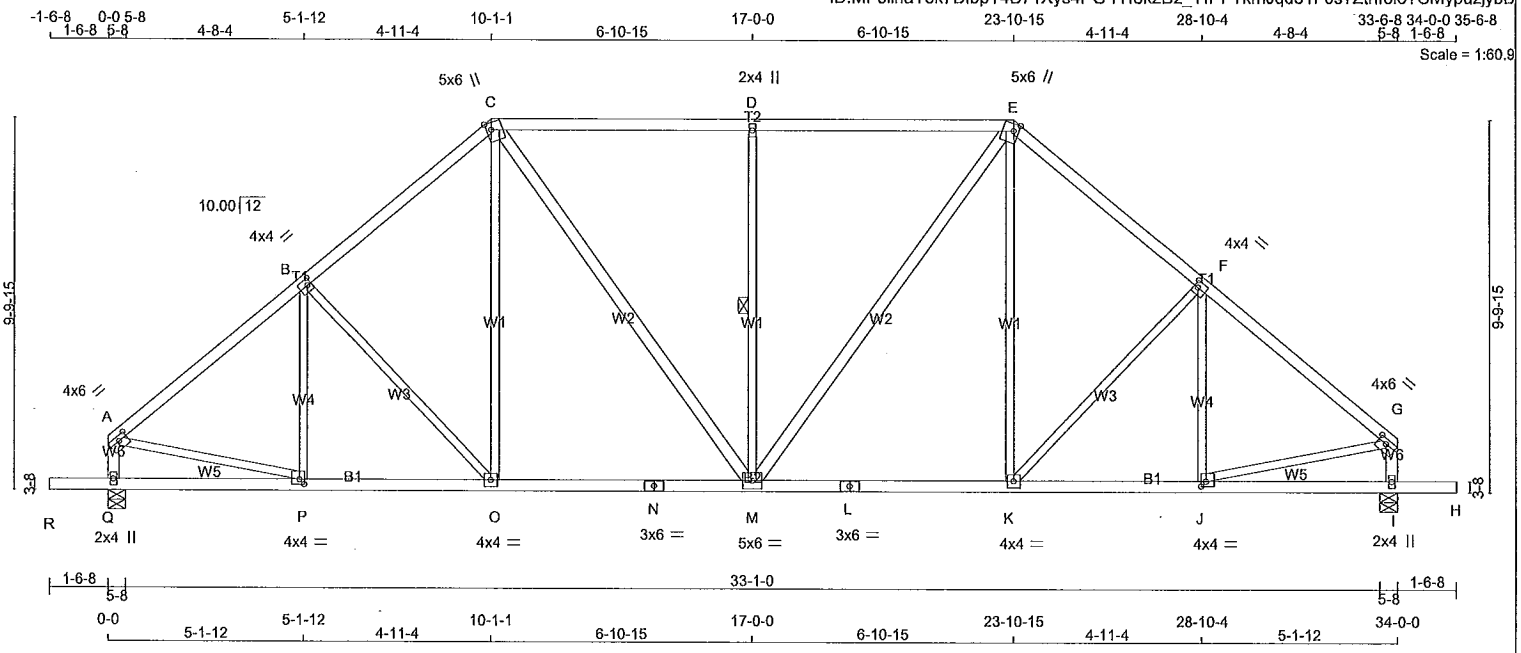


TOTAL WEIGHT = 5 X 167 = 835 lb

[M][F]

LUMBER

N. L. G. A. RULES

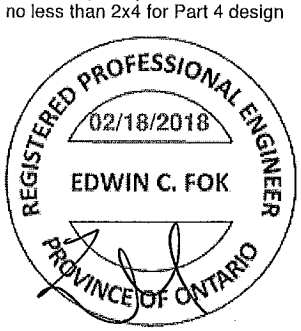
CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
Q - A	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
R - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - M	2x4	DRY	No.2	SPF
M - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0	1.50	2.75
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMWW-t	MT20	4.0	4.0	2.00	1.25
G	TMVW-t	MT20	4.0	6.0	1.50	2.75
I	BMV1+p	MT20	2.0	4.0		
J	BMWW-t	MT20	4.0	4.0	1.50	1.50
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	5.0	6.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	4.0	4.0	1.50	1.50
Q	BMV1+p	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	1841	0	1841	0	0	5-8	2-12
I	1841	0	1841	0	0	5-8	2-12

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1317	778 / 0	0 / 0	0 / 0	0 / 0	538 / 0	0 / 0
I	1317	778 / 0	0 / 0	0 / 0	0 / 0	538 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, I

BRACING

FOR SECTION C-E, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1835 / 0	-78.0	-78.0	0.32 (1)	4.66
B- C	-1702 / 0	-78.0	-78.0	0.30 (1)	4.82
C- D	-1595 / 0	-85.5	-85.5	0.58 (1)	2.00
D- E	-1595 / 0	-85.5	-85.5	0.58 (1)	2.00
E- F	-1702 / 0	-78.0	-78.0	0.30 (1)	4.82
F- G	-1835 / 0	-78.0	-78.0	0.32 (1)	4.66
Q- A	-1650 / 0	0.0	0.0	0.17 (1)	6.45
I- G	-1650 / 0	0.0	0.0	0.17 (1)	6.45
R- Q	0 / 0	-96.5	-96.5	0.16 (1)	10.00
Q- P	0 / 0	-18.5	-18.5	0.10 (4)	10.00
P- O	0 / 1432	-18.5	-18.5	0.30 (1)	10.00
O- N	0 / 1284	-18.5	-18.5	0.32 (4)	10.00
N- M	0 / 1284	-18.5	-18.5	0.32 (4)	10.00
M- L	0 / 1284	-18.5	-18.5	0.32 (4)	10.00
L- K	0 / 1284	-18.5	-18.5	0.32 (4)	10.00
K- J	0 / 1432	-18.5	-18.5	0.30 (1)	10.00
J- I	0 / 0	-18.5	-18.5	0.10 (4)	10.00
I- H	0 / 0	-96.5	-96.5	0.16 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- B	-206 / 0	0.10 (1)
B- O	-220 / 0	0.24 (1)
O- C	0 / 272	0.06 (1)
C- M	0 / 530	0.09 (1)
M- D	-725 / 0	0.47 (1)
M- E	0 / 530	0.09 (1)
K- E	0 / 272	0.06 (1)
K- F	-220 / 0	0.24 (1)
J- F	-206 / 0	0.10 (1)
A- P	0 / 1464	0.33 (1)
J- G	0 / 1464	0.33 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.13")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (1.13")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.58/1.00 (C-D:1) , BC=0.32/1.00 (M-O:4) , WB=0.47/1.00 (D-M:1) , SSI=0.29/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (P) (INPUT = 0.90)
JSI METAL= 0.54 (J) (INPUT = 1.00)

A-18023631

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292613	H2A	1	2	TRUSS DESC.	

Alpa Roof Truss, Maple

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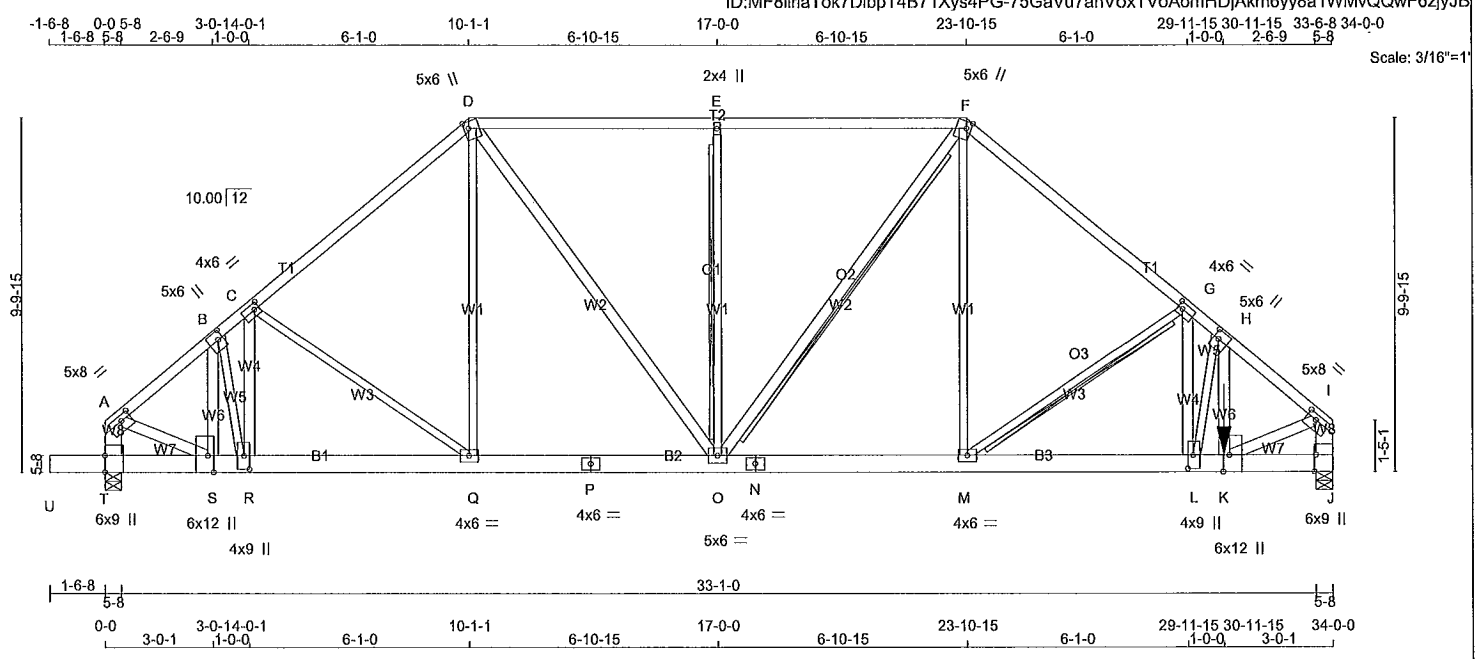


TOTAL WEIGHT = 2 X 201 = 402 lb

{M}

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
T - A	2x6	DRY	No.2	SPF
J - I	2x6	DRY	No.2	SPF
U - P	2x6	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
N - J	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
B - R	2x3	DRY	No.2	SPF
C - Q	2x3	DRY	No.2	SPF
Q - D	2x3	DRY	No.2	SPF
M - F	2x3	DRY	No.2	SPF
M - G	2x3	DRY	No.2	SPF
L - H	2x3	DRY	No.2	SPF
O - E	2x3	DRY	No.2	SPF

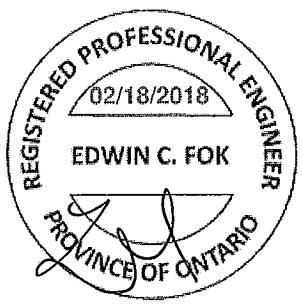
DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	TOP
D- F	1	12	TOP
F- I	1	12	TOP
T- A	2	12	TOP
J- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
U- P	2	12	TOP
P- N	2	12	TOP
N- J	2	12	SIDE(147.8)
WEBS : (0.122"X3") SPIRAL NAILS			
L- H	1	2	SIDE(752.3)
2x3	1	6	
H- K	1	2	SIDE(675.3)
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
T	2383	0	2383	0	0	5-8	1-8
J	7422	0	7422	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1694	1063 / 0	0 / 0	0 / 0	0 / 0	631 / 0	0 / 0
J	5196	3708 / 0	0 / 0	0 / 0	0 / 0	1489 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, J

BEARING SIZE FACTOR = 1.15 AT JNT(S) J (BASED ON SUPPORT DEPTH = 1-8)

BRACING

FOR SECTION D-F, MAX. PURLIN SPACING = 2.00 FT.

FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.31 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT G-M, E-O

2x6 DRY SPF No.2 T-BRACE AT F-O

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-2331 / 0	-78.0	-78.0	0.08 (1)	5.80
B- C	-2338 / 0	-78.0	-78.0	0.24 (1)	5.56
C- D	-2477 / 0	-78.0	-78.0	0.31 (1)	5.44
D- E	-2584 / 0	-85.5	-85.5	0.46 (1)	2.00
E- F	-2584 / 0	-85.5	-85.5	0.46 (1)	2.00
F- G	-3485 / 0	-78.0	-78.0	0.41 (1)	4.69
G- H	-6725 / 0	-78.0	-78.0	0.38 (1)	3.39
H- I	-7605 / 0	-78.0	-78.0	0.35 (1)	3.31
T- A	-2208 / 0	0.0	0.0	0.08 (1)	7.81
J- I	-6956 / 0	0.0	0.0	0.25 (1)	5.66
U- T	0 / 0	-96.5	-96.5	0.04 (1)	10.00
T- S	0 / 0	-18.5	-18.5	0.01 (4)	10.00
S- R	0 / 1830	-18.5	-18.5	0.13 (1)	10.00
R- Q	0 / 1945	-18.5	-18.5	0.15 (1)	10.00
Q- P	0 / 1878	-18.5	-18.5	0.15 (1)	10.00
P- O	0 / 1878	-18.5	-18.5	0.15 (1)	10.00
O- N	0 / 2667	-18.5	-18.5	0.20 (1)	10.00
N- M	0 / 2667	-18.5	-18.5	0.20 (1)	10.00
M- L	0 / 5234	-18.5	-18.5	0.39 (1)	10.00
L- K	0 / 5819	-18.5	-18.5	0.71 (1)	10.00
K- J	0 / 0	-108.9	-108.9	0.33 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- B	-530 / 0	0.05 (1)
B- R	0 / 549	0.07 (1)
R- C	-520 / 0	0.06 (1)
C- Q	-84 / 0	0.05 (1)
Q- D	0 / 186	0.03 (4)
M- F	0 / 1912	0.24 (1)
M- G	-3161 / 0	0.88 (1)
L- G	0 / 3302	0.29 (1)
L- H	-1930 / 0	0.25 (1)
K- H	0 / 2597	0.23 (1)
A- S	0 / 1897	0.17 (1)
K- I	0 / 6134	0.54 (1)
O- F	-139 / 60	0.03 (1)
D- O	0 / 1196	0.11 (1)
O- E	-718 / 0	0.35 (1)

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

GIRDER TYPE: CStdGirder
START DISTANCE = 30-11-14
START SPAN CARRIED = 3-10-8
END DISTANCE = 34-0-0
END SPAN CARRIED = 3-10-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

GIRDER TYPE: CStdGirder
START DISTANCE = 30-11-14
START SPAN CARRIED = 3-10-8
END DISTANCE = 34-0-0
END SPAN CARRIED = 3-10-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.13")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (1.13")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

A-18023632

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292613	H2A	1	2	TRUSS DESC.	

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

PLATES (table is in inches)

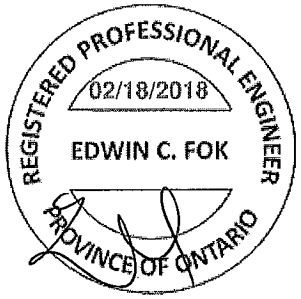
JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	8.0	1.75	3.25
B	TMWW+t	MT20	5.0	6.0	2.75	1.75
C	TMWW-t	MT20	4.0	6.0	2.00	1.75
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	4.0	6.0	2.00	1.75
H	TMWW+t	MT20	5.0	6.0	2.75	1.75
I	TMVW-t	MT20	5.0	8.0	1.75	3.25
J	BMV1+t	MT20	6.0	9.0	Edge	0.50
K	BMWW+t	MT20	6.0	12.0	Edge	2.00
L	BMWW+t	MT20	4.0	9.0	4.50	1.75
M	BMWW-t	MT20	4.0	6.0		
N	BS-t	MT20	4.0	6.0		
O	BMWWW-t	MT20	5.0	6.0		
P	BS-t	MT20	4.0	6.0		
Q	BMWW-t	MT20	4.0	6.0		
R	BMWW+t	MT20	4.0	9.0	4.50	1.75
S	BMWW+t	MT20	6.0	12.0	Edge	2.00
T	BMV1+t	MT20	6.0	9.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 6001,3 lbs FACTORED DOWN AT 30-11-14 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
K	30-11-14	-6001	-6001	---	FRONT	VERT	TOTAL

CSI: TC=0.46/1.00 (D-E:1) , BC=0.71/1.00 (K-L:1) , WB=0.88/1.00 (G-M:1) , SSI=0.44/1.00 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (S) (INPUT = 0.90)
JSI METAL= 0.85 (S) (INPUT = 1.00)

A:18023632(2)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292613	H3	7	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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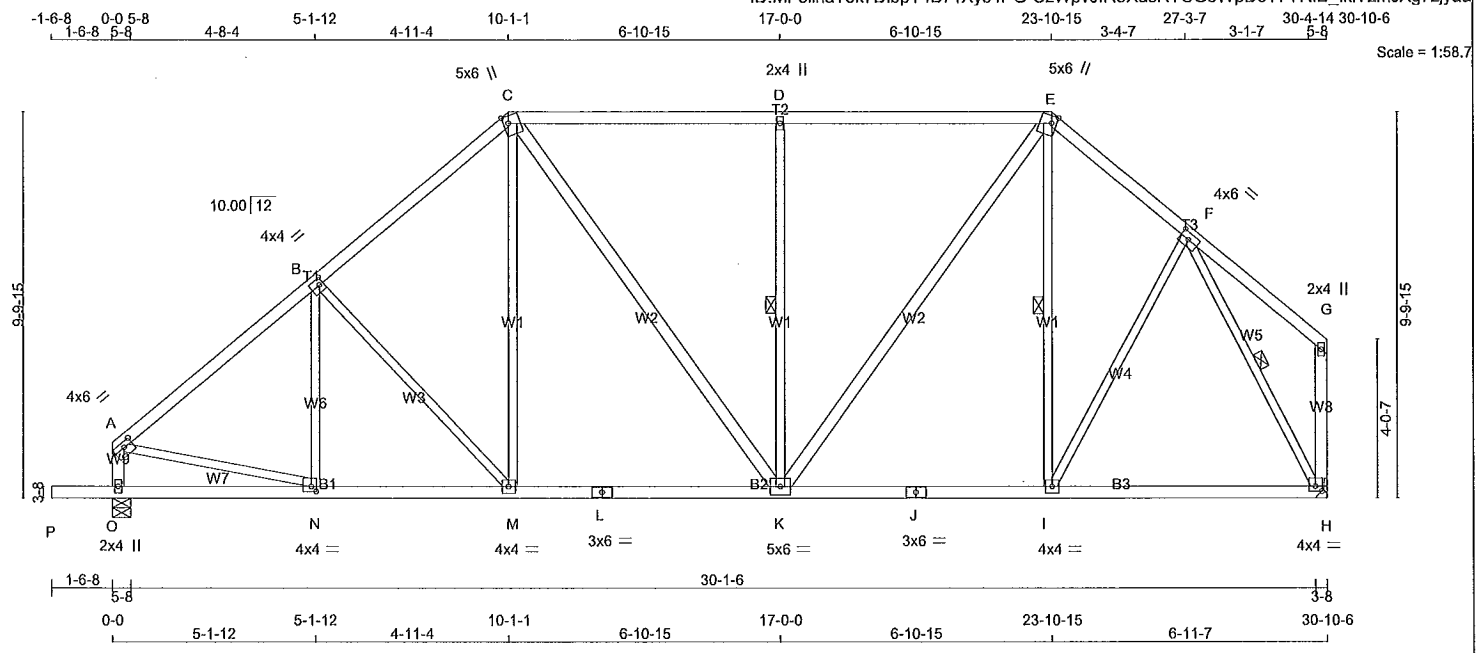


TOTAL WEIGHT = 7 X 158 = 1105 lb

[M][F]

LUMBER

N. L. G. A. RULES

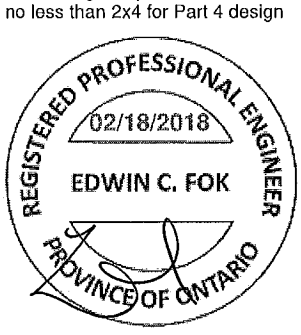
CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
O - A	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
P - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
C - K	2x4	DRY	No.2	SPF
K - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0	1.50	2.75
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMWW-t	MT20	4.0	6.0	2.00	2.75
G	TMV+p	MT20	2.0	4.0		
H	BMVW1-t	MT20	4.0	4.0	1.50	2.00
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	5.0	6.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMWW-t	MT20	4.0	4.0	1.50	1.50
O	BMV1+p	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
O	1684	0	1684	0	0	5-8	2-8
H	1546	0	1546	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-11	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1205	713 / 0	0 / 0	0 / 0	0 / 0	492 / 0	0 / 0
H	1107	648 / 0	0 / 0	0 / 0	0 / 0	459 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING

FOR SECTION C-E, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-K, E-I, F-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-1640 / 0	-78.0	-78.0	0.31 (1)	4.88	N- B	-175 / 0	0.09 (1)
B- C	-1486 / 0	-78.0	-78.0	0.29 (1)	5.08	B- M	-244 / 0	0.26 (1)
C- D	-1315 / 0	-85.5	-85.5	0.56 (1)	2.00	M- C	0 / 292	0.07 (1)
D- E	-1315 / 0	-85.5	-85.5	0.56 (1)	2.00	C- K	0 / 335	0.05 (1)
E- F	-1191 / 0	-78.0	-78.0	0.13 (1)	5.73	K- D	-727 / 0	0.47 (1)
F- G	0 / 19	-78.0	-78.0	0.15 (1)	10.00	K- E	0 / 716	0.12 (1)
O- A	-1494 / 0	0.0	0.0	0.15 (1)	6.71	I- E	-157 / 0	0.10 (1)
H- G	-107 / 0	0.0	0.0	0.03 (1)	7.81	I- F	0 / 343	0.08 (1)
						A- N	0 / 1311	0.29 (1)
P- O	0 / 0	-96.5	-96.5	0.16 (1)	10.00	F- H	-1569 / 0	0.57 (1)
O- N	0 / 0	-18.5	-18.5	0.10 (4)	10.00			
N- M	0 / 1283	-18.5	-18.5	0.28 (1)	10.00			
M- L	0 / 1119	-18.5	-18.5	0.28 (4)	10.00			
L- K	0 / 1119	-18.5	-18.5	0.28 (4)	10.00			
K- J	0 / 895	-18.5	-18.5	0.31 (4)	10.00			
J- I	0 / 895	-18.5	-18.5	0.31 (4)	10.00			
I- H	0 / 735	-18.5	-18.5	0.29 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.56/1.00 (C-D:1) , BC=0.31/1.00 (I-K:4) , WB=0.57/1.00 (F-H:1) , SSI=0.29/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (H) (INPUT = 0.90)
JSI METAL= 0.48 (N) (INPUT = 1.00)

A-18023633

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292613	H4P	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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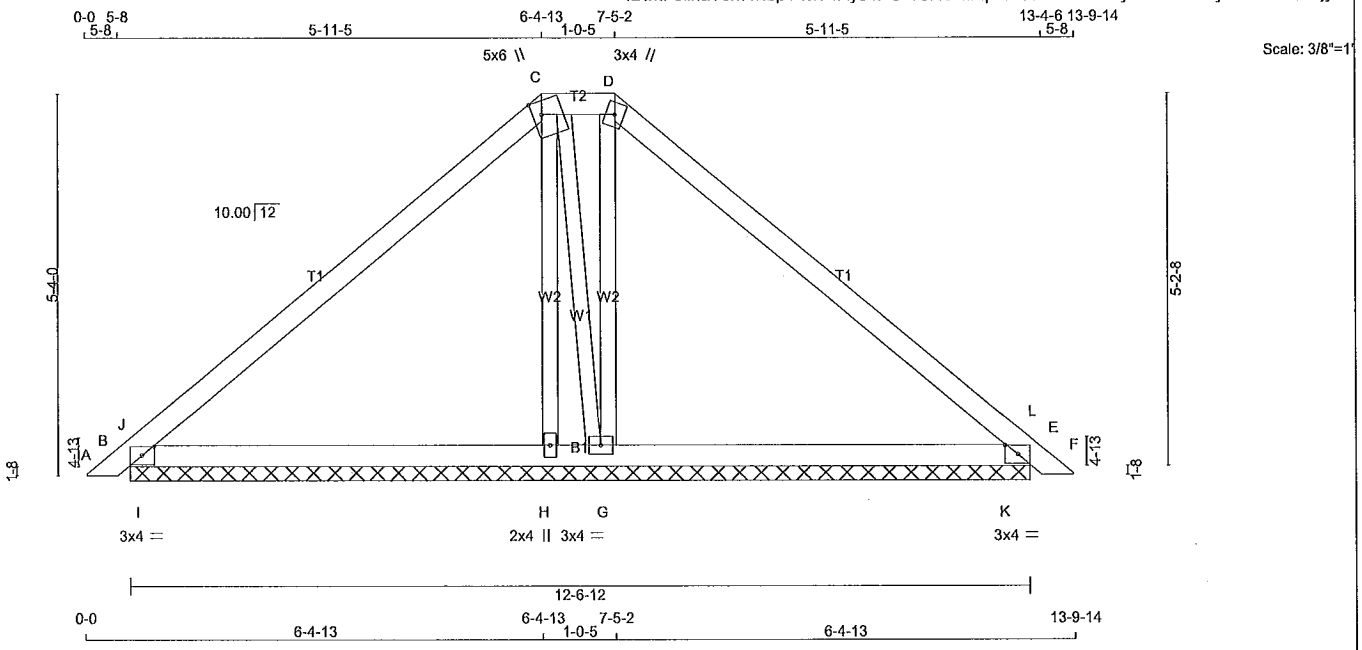


TOTAL WEIGHT = 46 lb

[M]

LUMBER

N. L. G. A. RULES

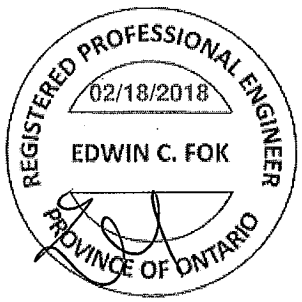
CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTWW+m	MT20	5:0	6.0	2.25	1.50
D	TTW+m	MT20	3.0	4.0		
E	TMB1-I	MT20	3.0	4.0	1.50	2.00
G	BMWW1-I	MT20	3.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	453	0	453	0	0	12-6-12	9-0
E	448	0	448	0	0	12-6-12	9-0
H	158	0	158	0	0	12-6-12	9-0
G	233	0	233	0	0	12-6-12	9-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	321	210 / 0	0 / 0	0 / 0	0 / 0	110 / 0	0 / 0
E	317	207 / 0	0 / 0	0 / 0	0 / 0	110 / 0	0 / 0
H	116	53 / 0	0 / 0	0 / 0	0 / 0	63 / 0	0 / 0
G	167	101 / 0	0 / 0	0 / 0	0 / 0	66 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 12	-78.0	-78.0	0.02 (1)	10.00	H- C	-41 / 0		0.02 (1)
B- J	-61 / 137	-78.0	-78.0	0.21 (1)	6.25	C- G	-36 / 0		0.01 (1)
J- C	-257 / 0	-78.0	-78.0	0.33 (1)	6.25	G- D	-81 / 0		0.03 (1)
C- D	-178 / 0	-78.0	-78.0	0.01 (1)	6.25	I- J	-758 / 0		0.00 (1)
D- L	-250 / 0	-78.0	-78.0	0.33 (1)	6.25	K- L	-759 / 0		0.00 (1)
L- E	-60 / 145	-78.0	-78.0	0.21 (1)	6.25				
E- F	0 / 12	-78.0	-78.0	0.02 (1)	10.00				
B- I	0 / 185	-18.5	-18.5	0.28 (1)	10.00				
I- H	0 / 185	-18.5	-18.5	0.28 (1)	10.00				
H- G	0 / 184	-18.5	-18.5	0.22 (1)	10.00				
G- K	0 / 179	-18.5	-18.5	0.28 (1)	10.00				
K- E	0 / 179	-18.5	-18.5	0.28 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.33/1.00 (C-J:1) , BC=0.28/1.00 (B-I:1) , WB=0.03/1.00 (D-G:1) , SSI=0.59/1.00 (E-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.39 (B) (INPUT = 0.90)
JSI METAL= 0.11 (B) (INPUT = 1.00)

A-18023634

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292613	H5P	11	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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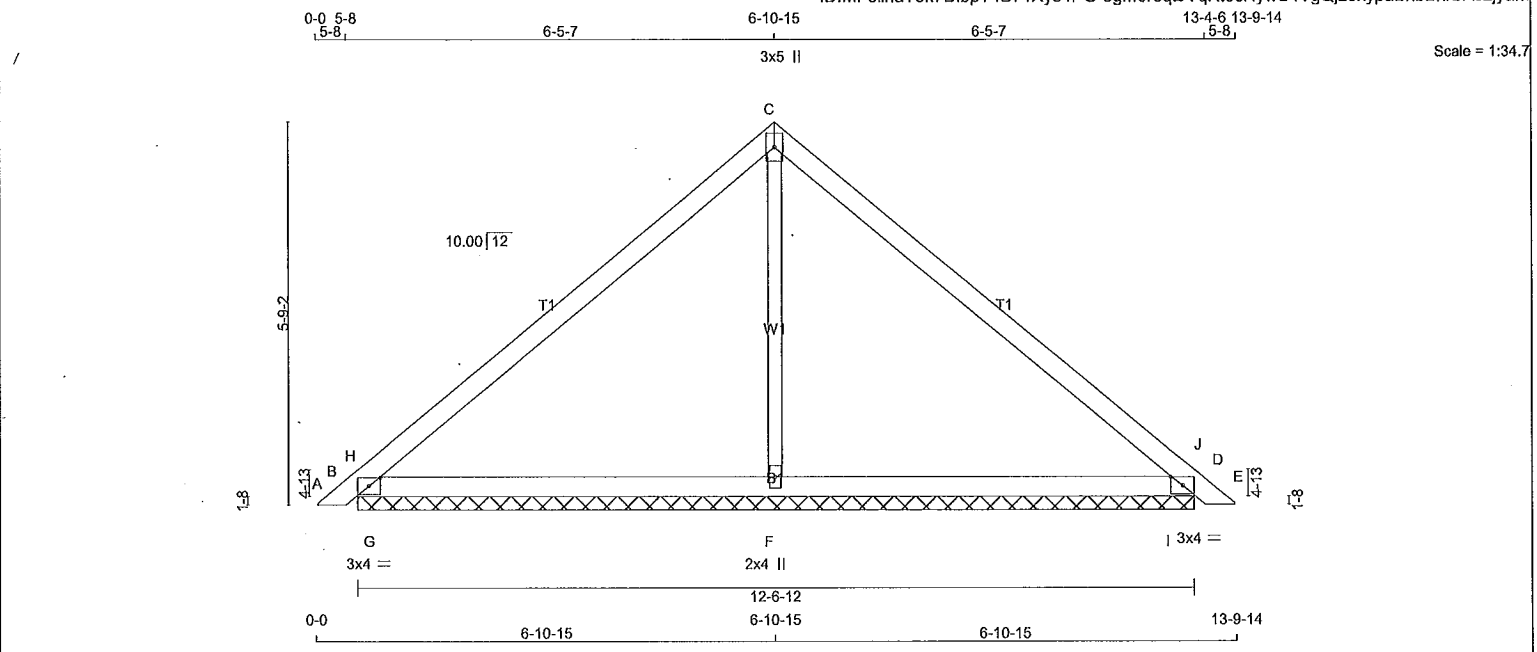


TOTAL WEIGHT = 11 X 39 = 427 lb [M]

LUMBER
N. L. G. A. RULES

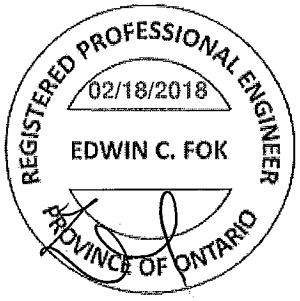
CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTW+p	MT20	3.0	5.0		
D	TMB1-I	MT20	3.0	4.0	1.50	2.00
F	BMW1+w	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	FACTORED GROSS REACTION HORZ	MAXIMUM FACTORED DOWN	MAXIMUM FACTORED HORZ	MAXIMUM FACTORED UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	491	0	491	0	0	12-6-12	1-5-11
D	491	0	491	0	0	12-6-12	1-5-11
F	312	0	312	0	0	12-6-12	1-5-11

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	346	231 / 0	0 / 0	0 / 0	0 / 0	115 / 0	0 / 0
D	346	231 / 0	0 / 0	0 / 0	0 / 0	115 / 0	0 / 0
F	228	109 / 0	0 / 0	0 / 0	0 / 0	119 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 12	-78.0	-78.0	0.02 (1)	10.00	F- C	-57 / 9	0.03 (1)
B- H	-40 / 219	-78.0	-78.0	0.28 (1)	6.25	G- H	-942 / 0	0.00 (1)
H- C	-296 / 0	-78.0	-78.0	0.39 (1)	6.25	I- J	-941 / 0	0.00 (1)
C- J	-296 / 0	-78.0	-78.0	0.39 (1)	6.25			
J- D	-40 / 219	-78.0	-78.0	0.28 (1)	6.25			
D- E	0 / 12	-78.0	-78.0	0.02 (1)	10.00			
B- G	0 / 213	-18.5	-18.5	0.34 (1)	10.00			
G- F	0 / 213	-18.5	-18.5	0.34 (1)	10.00			
F- I	0 / 213	-18.5	-18.5	0.34 (1)	10.00			
I- D	0 / 213	-18.5	-18.5	0.34 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.39/1.00 (C-H:1) , BC=0.34/1.00 (B-G:1) , WB=0.03/1.00 (C-F:1) , SSI=0.73/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.43 (B) (INPUT = 0.90)
JSI METAL= 0.12 (D) (INPUT = 1.00)

A-18023635

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292613	H6	3	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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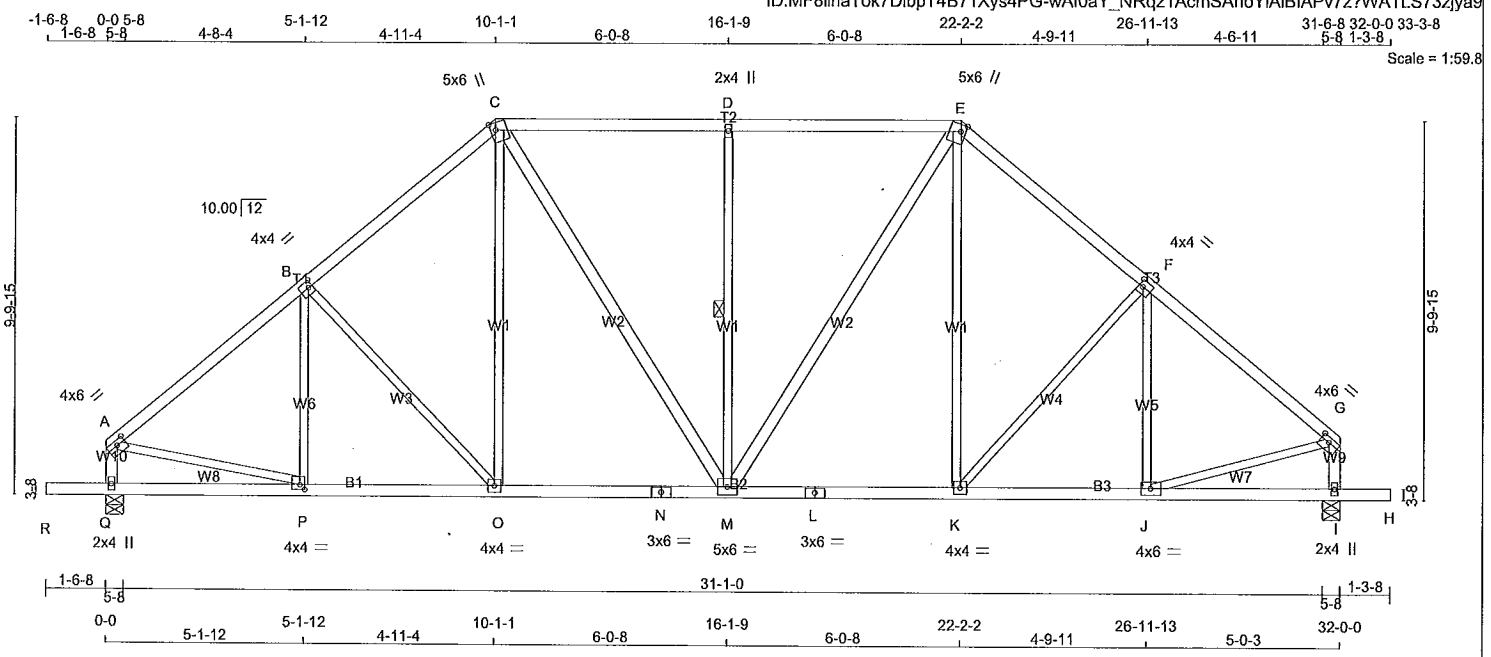


TOTAL WEIGHT = 3 X 161 = 484 lb

[M][F]

LUMBER
N. L. G. A. RULES

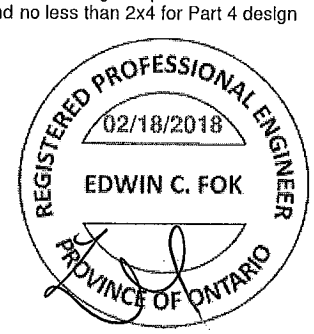
CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
Q - A	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
R - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - M	2x4	DRY	No.2	SPF
M - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0	1.50	2.75
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMWW-t	MT20	4.0	4.0	2.00	1.25
G	TMVW-t	MT20	4.0	6.0	1.50	2.75
I	BMV1+p	MT20	2.0	4.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	5.0	6.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	4.0	4.0	1.50	1.50
Q	BMV1+p	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
Q	1737	0	1737	0	0	5-8	2-9
I	1714	0	1714	0	0	5-8	2-9

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1242	736 / 0	0 / 0	0 / 0	0 / 0	506 / 0	0 / 0
I	1226	726 / 0	0 / 0	0 / 0	0 / 0	500 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, I

BRACING
FOR SECTION C-E, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.80 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1707 / 0	-78.0	-78.0	0.31 (1)	4.80
B- C	-1558 / 0	-78.0	-78.0	0.29 (1)	4.99
C- D	-1402 / 0	-85.5	-85.5	0.43 (1)	2.00
D- E	-1402 / 0	-85.5	-85.5	0.43 (1)	2.00
E- F	-1536 / 0	-78.0	-78.0	0.28 (1)	5.04
F- G	-1636 / 0	-78.0	-78.0	0.29 (1)	4.91
Q- A	-1548 / 0	0.0	0.0	0.16 (1)	6.62
I- G	-1550 / 0	0.0	0.0	0.16 (1)	6.61
R- Q	0 / 0	-96.5	-96.5	0.16 (1)	10.00
Q- P	0 / 0	-18.5	-18.5	0.10 (4)	10.00
P- O	0 / 1334	-18.5	-18.5	0.27 (1)	10.00
O- N	0 / 1173	-18.5	-18.5	0.26 (1)	10.00
N- M	0 / 1173	-18.5	-18.5	0.26 (1)	10.00
M- L	0 / 1157	-18.5	-18.5	0.26 (1)	10.00
L- K	0 / 1157	-18.5	-18.5	0.26 (1)	10.00
K- J	0 / 1279	-18.5	-18.5	0.26 (1)	10.00
J- I	0 / 0	-18.5	-18.5	0.10 (4)	10.00
I- H	0 / 0	-96.5	-96.5	0.11 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- B	-182 / 0	0.09 (1)
B- O	-239 / 0	0.25 (1)
O- C	0 / 276	0.06 (1)
C- M	0 / 426	0.07 (1)
M- D	-632 / 0	0.41 (1)
M- E	0 / 457	0.07 (1)
K- E	0 / 240	0.05 (1)
K- F	-187 / 0	0.20 (1)
J- F	-236 / 0	0.12 (1)
A- P	0 / 1363	0.31 (1)
J- G	0 / 1322	0.30 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	21.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.07")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (1.07")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.43/1.00 (C-D:1) , BC=0.27/1.00 (O-P:1) , WB=0.41/1.00 (D-M:1) , SSI=0.25/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (J) (INPUT = 0.90)
JSI METAL= 0.50 (P) (INPUT = 1.00)

A-18023636

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292613	H6A	1	2	TRUSS DESC.	

Alpa Roof Truss, Maple

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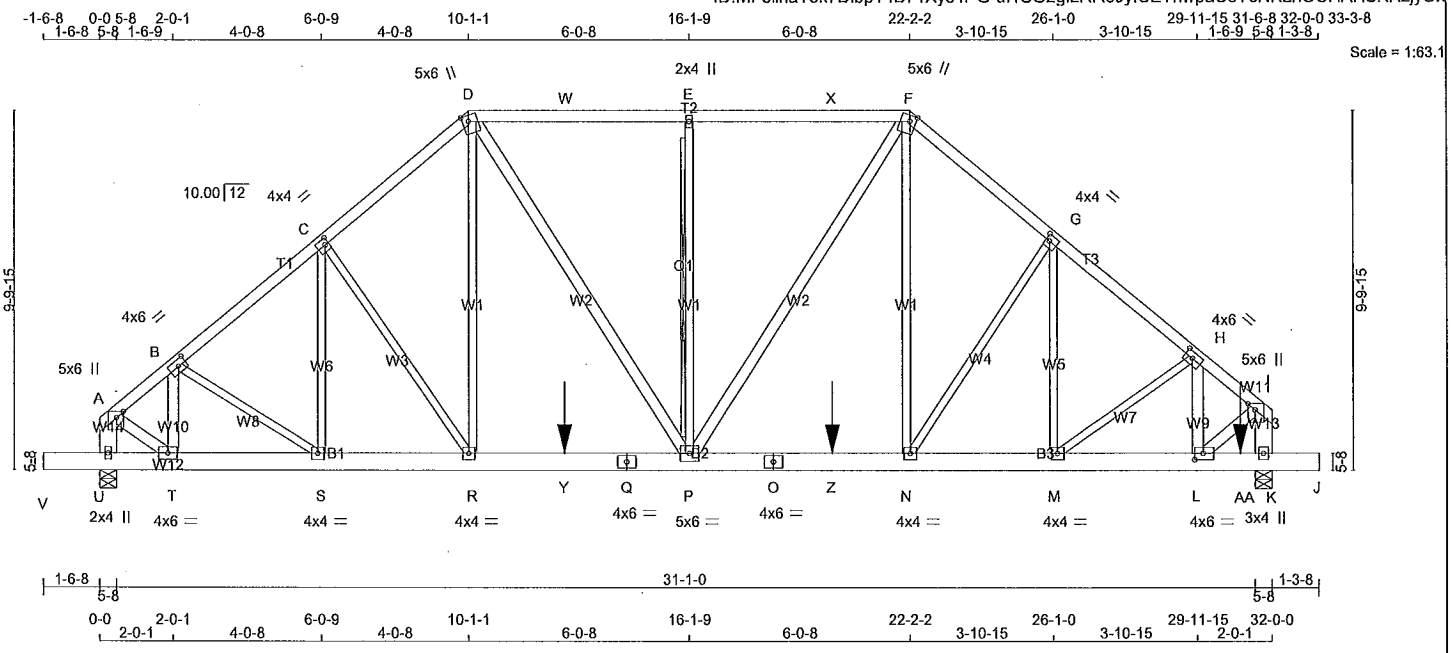


TOTAL WEIGHT = 2 X 195 = 389 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
U - A	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
V - Q	2x6	DRY	No.2	SPF
Q - O	2x6	DRY	No.2	SPF
O - J	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
D - P	2x4	DRY	No.2	SPF
P - F	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
A - T	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

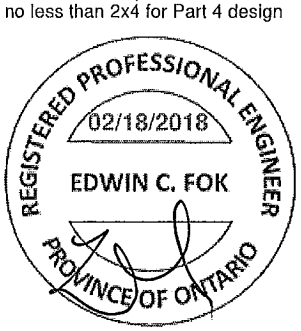
DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	TOP
D- F	1	12	SIDE(38.2)
F- I	1	12	TOP
U- A	2	12	TOP
K- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
V- Q	2	12	SIDE(10.5)
Q- O	2	12	SIDE(10.5)
O- J	2	12	SIDE(10.5)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
U	3197	0	3197	0	0	5-8	1-11
K	5437	0	5437	0	0	5-8	3-2

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	2258	1496 / 0	0 / 0	0 / 0	0 / 0	763 / 0	0 / 0
K	3815	2677 / 0	0 / 0	0 / 0	0 / 0	1138 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.36 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT E-P

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-2834 / 0	-78.0	-78.0	0.07 (1)		5.40
B- C	-3544 / 0	-78.0	-78.0	0.16 (1)		4.86
C- D	-3673 / 0	-78.0	-78.0	0.16 (1)		4.79
D- W	-3491 / 0	-78.0	-78.0	0.55 (1)		4.36
W- E	-3491 / 0	-154.5	-154.5	0.55 (1)		4.36
E- X	-3491 / 0	-154.5	-154.5	0.55 (1)		4.36
X- F	-3491 / 0	-78.0	-78.0	0.55 (1)		4.36
F- G	-3632 / 0	-78.0	-78.0	0.15 (1)		4.82
G- H	-3664 / 0	-78.0	-78.0	0.15 (1)		4.80
H- I	-3556 / 0	-78.0	-78.0	0.08 (1)		4.93
U- A	-2970 / 0	0.0	0.0	0.11 (1)		7.81
K- I	-4012 / 0	0.0	0.0	0.15 (1)		7.10
V- U	0 / 0	-96.5	-96.5	0.04 (1)		10.00
U- T	0 / 0	-18.5	-18.5	0.04 (1)		10.00
T- S	0 / 2192	-18.5	-18.5	0.21 (1)		10.00
S- R	0 / 2730	-18.5	-18.5	0.43 (1)		10.00
R- Y	0 / 2809	-18.5	-18.5	0.63 (1)		10.00
Y- Q	0 / 2809	-37.3	-37.3	0.63 (1)		10.00
Q- P	0 / 2809	-37.3	-37.3	0.63 (1)		10.00
P- O	0 / 2777	-37.3	-37.3	0.50 (1)		10.00
O- Z	0 / 2777	-37.3	-37.3	0.50 (1)		10.00
Z- N	0 / 2777	-18.5	-18.5	0.50 (1)		10.00
N- M	0 / 2824	-18.5	-18.5	0.33 (1)		10.00
M- L	0 / 2744	-18.5	-18.5	0.26 (1)		10.00
L- AA	0 / 0	-18.5	-18.5	0.41 (1)		10.00
AA- K	0 / 0	-18.5	-18.5	0.41 (1)		10.00
K- J	0 / 0	-96.5	-96.5	0.03 (1)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- B	-1055 / 0	0.07 (1)
S- C	-535 / 0	0.17 (1)
C- R	0 / 122	0.02 (1)
R- D	0 / 931	0.12 (1)
D- P	0 / 1264	0.11 (1)
P- E	-1027 / 0	0.50 (1)
P- F	0 / 1324	0.12 (1)
N- F	0 / 836	0.10 (1)
N- G	-103 / 0	0.05 (1)
M- G	-236 / 0	0.08 (1)
M- H	0 / 100	0.01 (4)
L- H	-362 / 0	0.03 (1)
A- T	0 / 2451	0.22 (1)
L- I	0 / 3213	0.28 (1)

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.07")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (1.07")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.55/1.00 (E-F:1) , BC=0.63/1.00 (P-R:1) , WB=0.50/1.00 (E-P:1) , SSI=0.43/1.00 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

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CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292613	H6A	1	2	TRUSS DESC.	

Alpa Roof Truss, Maple

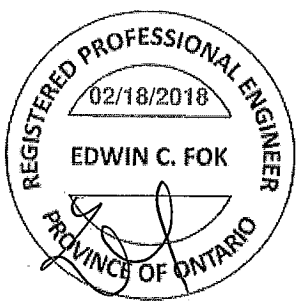
PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	2.00	2.25
B	TMWW-t	MT20	4.0	6.0	2.00	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.00	2.00
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.00	2.00
G	TMWW-t	MT20	4.0	4.0	2.00	1.25
H	TMWW-t	MT20	4.0	6.0	2.00	2.75
I	TMVW+p	MT20	5.0	6.0	2.00	2.25
K	BMV1+p	MT20	3.0	4.0		
L	BMWW-t	MT20	4.0	6.0	2.00	2.75
M, N, R, S						
M	BMWW-t	MT20	4.0	4.0		
O	BS-t	MT20	4.0	6.0		
P	BMWWW-t	MT20	5.0	6.0		
Q	BS-t	MT20	4.0	6.0		
T	BMWW-t	MT20	4.0	6.0		
U	BMV1+p	MT20	2.0	4.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1282.6 lbs FACTORED DOWN AT 12-8-8, AND 860.8 lbs FACTORED DOWN AT 20-0-6, AND 2432.8 lbs FACTORED DOWN AT 31-1-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
Y	12-8-8	-1283	-1283	---	FRONT	VERT	TOTAL
Z	20-0-6	-861	-861	---	FRONT	VERT	TOTAL
AA	31-1-12	-2433	-2433	---	FRONT	VERT	TOTAL

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (L) (INPUT = 0.90)
JSI METAL= 0.47 (Q) (INPUT = 1.00)

A-18023637(2)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292613	H6B	1	2	TRUSS DESC.	

Alpa Roof Truss, Maple

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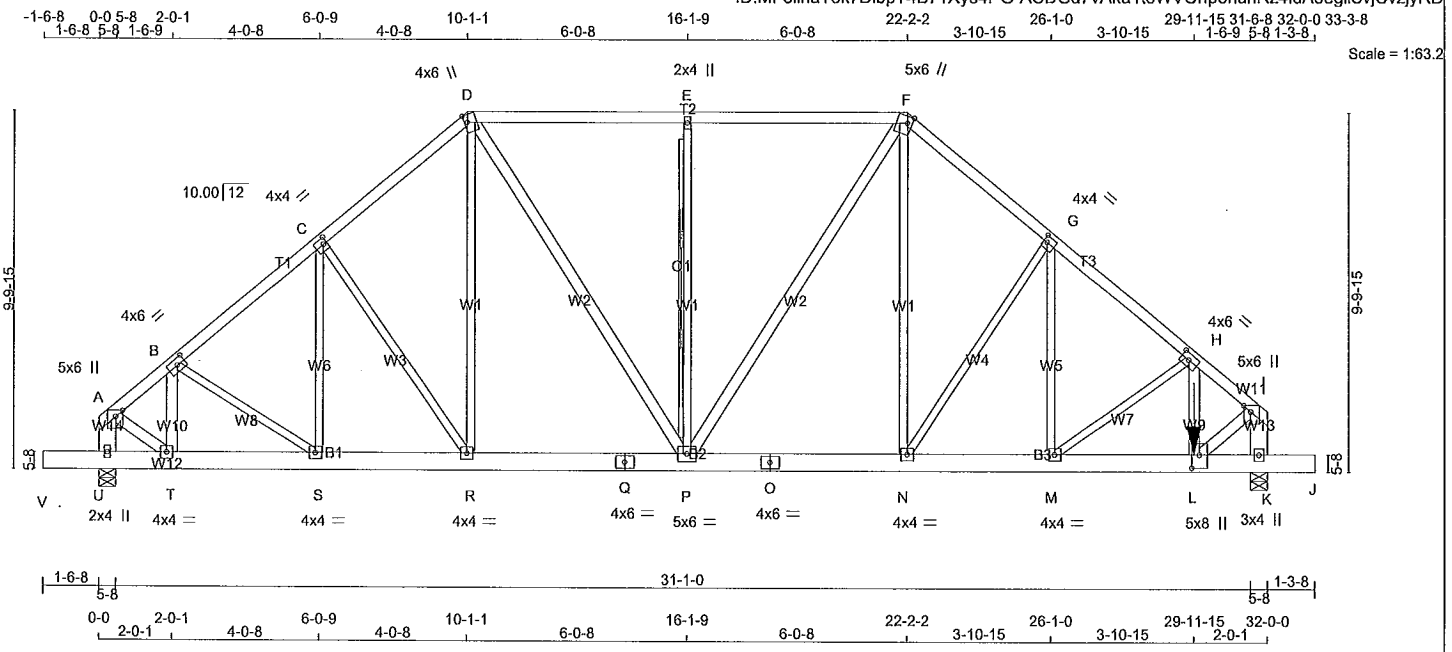


TOTAL WEIGHT = 2 X 195 = 389 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
U - A	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
V - Q	2x6	DRY	No.2	SPF
Q - O	2x6	DRY	No.2	SPF
O - J	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
D - P	2x4	DRY	No.2	SPF
P - F	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
A - T	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF

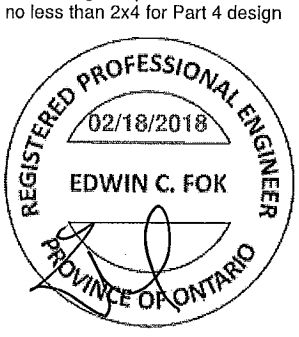
DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	TOP
D- F	1	12	TOP
F- I	1	12	TOP
U- A	2	12	TOP
K- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
V- Q	2	12	TOP
Q- O	2	12	TOP
O- J	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
H- L	2	3	SIDE(774.9)
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
U	1971	0	1971	0	0	5-8	1-8
K	5152	0	5152	0	0	5-8	2-12

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1405	859 / 0	0 / 0	0 / 0	0 / 0	546 / 0	0 / 0
K	3614	2537 / 0	0 / 0	0 / 0	0 / 0	1078 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, K

BRACING

FOR SECTION D-F, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.55 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT E-P

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1623 / 0	-78.0	-78.0	0.06 (1)	6.25
B- C	-1986 / 0	-78.0	-78.0	0.12 (1)	6.11
C- D	-1872 / 0	-78.0	-78.0	0.12 (1)	6.24
D- E	-1800 / 0	-85.5	-85.5	0.32 (1)	2.00
E- F	-1800 / 0	-85.5	-85.5	0.32 (1)	2.00
F- G	-2231 / 0	-78.0	-78.0	0.12 (1)	5.85
G- H	-2911 / 0	-78.0	-78.0	0.14 (1)	5.27
H- I	-4338 / 0	-78.0	-78.0	0.08 (1)	4.55
U- A	-1730 / 0	0.0	0.0	0.06 (1)	7.81
K- I	-4882 / 0	0.0	0.0	0.18 (1)	6.57
V- U	0 / 0	-96.5	-96.5	0.04 (1)	10.00
U- T	0 / 0	-18.5	-18.5	0.04 (1)	10.00
T- S	0 / 1263	-18.5	-18.5	0.10 (1)	10.00
S- R	0 / 1534	-18.5	-18.5	0.11 (1)	10.00
R- Q	0 / 1421	-18.5	-18.5	0.11 (1)	10.00
Q- P	0 / 1421	-18.5	-18.5	0.11 (1)	10.00
P- O	0 / 1704	-18.5	-18.5	0.13 (1)	10.00
O- N	0 / 1704	-18.5	-18.5	0.13 (1)	10.00
N- M	0 / 2249	-18.5	-18.5	0.16 (1)	10.00
M- L	0 / 3339	-18.5	-18.5	0.31 (1)	10.00
L- K	0 / 0	-18.5	-18.5	0.09 (1)	10.00
K- J	0 / 0	-96.5	-96.5	0.03 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- B	-656 / 0	0.04 (1)
B- S	0 / 325	0.04 (1)
S- C	-108 / 13	0.03 (1)
C- R	-205 / 0	0.11 (1)
R- D	0 / 271	0.03 (1)
D- P	0 / 703	0.06 (1)
P- E	-629 / 0	0.30 (1)
P- F	0 / 179	0.02 (4)
N- F	0 / 948	0.12 (1)
N- G	-1018 / 0	0.53 (1)
M- G	0 / 942	0.12 (1)
M- H	-1355 / 0	0.27 (1)
L- H	0 / 1496	0.13 (1)
A- T	0 / 1412	0.12 (1)
L- I	0 / 3910	0.35 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
L	29-11-14	-3672	-3672	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.07")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (1.07")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.32/1.00 (E-F:1) , BC=0.31/1.00 (L-M:1) , WB=0.53/1.00 (G-N:1) , SSI=0.14/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

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CONTINUED ON PAGE 2

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

PLATES (table is in inches)

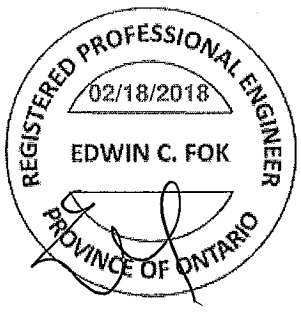
JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	2.00	2.25
B	TMWW-t	MT20	4.0	6.0	2.00	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	4.0	6.0	Edge	1.00
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	4.0	4.0	2.00	1.25
H	TMWW-t	MT20	4.0	6.0	2.00	2.75
I	TMVW+p	MT20	5.0	6.0	2.00	2.25
K	BMV1+p	MT20	3.0	4.0		
L	BMWW+t	MT20	5.0	8.0	4.25	2.50
M, N, R, S, T						
M	BMWW-t	MT20	4.0	4.0		
O	BS-t	MT20	4.0	6.0		
P	BMWWW-t	MT20	5.0	6.0		
Q	BS-t	MT20	4.0	6.0		
U	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 3672.2 lbs FACTORED DOWN AT 29-11-14 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (K) (INPUT = 0.90)
JSI METAL= 0.60 (L) (INPUT = 1.00)

A-180236880

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292613	H6S	4	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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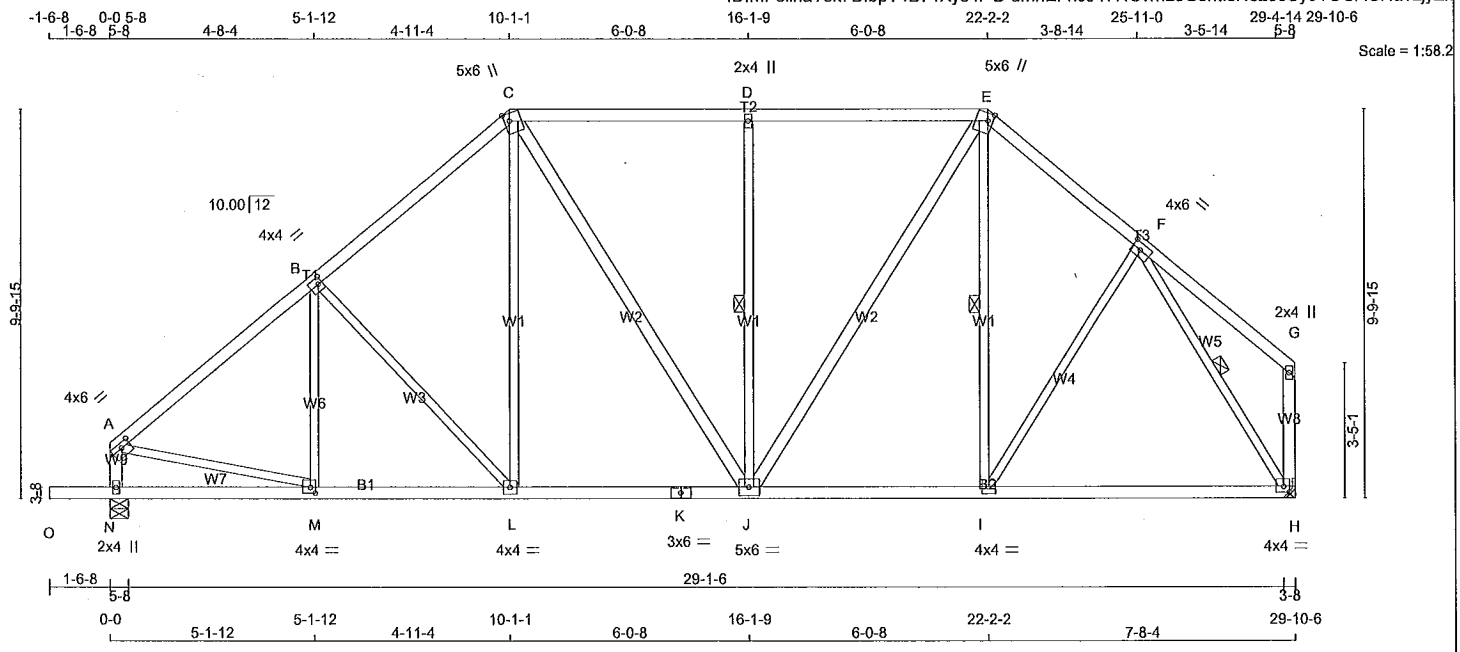


TOTAL WEIGHT = 4 X 154 = 615 lb

[M][F]

LUMBER

N. L. G. A. RULES

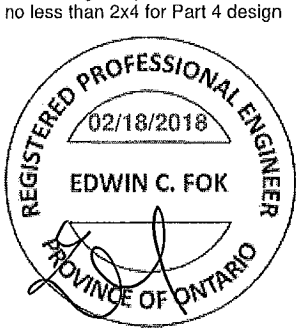
CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
N - A	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
C - J	2x4	DRY	No.2	SPF
J - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0	1.50	2.75
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMWW-t	MT20	4.0	6.0	2.00	2.75
G	TMV+p	MT20	2.0	4.0		
H	BMVW1-t	MT20	4.0	4.0	1.75	1.75
I	BMWW-t	MT20	4.0	4.0		
J	BMWWW-t	MT20	5.0	6.0		
K	BS-t	MT20	3.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMWW-t	MT20	4.0	4.0	1.75	1.50
N	BMV1+p	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
N	1631	0	1631	0	0	5-8	2-7
H	1489	0	1489	0	0	HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-10	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1166	692 / 0	0 / 0	0 / 0	0 / 0	475 / 0	0 / 0
H	1066	627 / 0	0 / 0	0 / 0	0 / 0	439 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

FOR SECTION C-E, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.96 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-J, E-I, F-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1574 / 0	-78.0	-78.0	0.30 (1)	4.96
B- C	-1411 / 0	-78.0	-78.0	0.29 (1)	5.19
C- D	-1220 / 0	-85.5	-85.5	0.42 (1)	2.00
D- E	-1220 / 0	-85.5	-85.5	0.42 (1)	2.00
E- F	-1218 / 0	-78.0	-78.0	0.16 (1)	5.65
F- G	0 / 22	-78.0	-78.0	0.18 (1)	10.00
N- A	-1441 / 0	0.0	0.0	0.15 (1)	6.81
H- G	-117 / 0	0.0	0.0	0.02 (1)	7.81
O- N	0 / 0	-96.5	-96.5	0.16 (1)	10.00
N- M	0 / 0	-18.5	-18.5	0.10 (4)	10.00
M- L	0 / 1232	-18.5	-18.5	0.26 (1)	10.00
L- K	0 / 1061	-18.5	-18.5	0.24 (1)	10.00
K- J	0 / 1061	-18.5	-18.5	0.24 (1)	10.00
J- I	0 / 915	-18.5	-18.5	0.34 (4)	10.00
I- H	0 / 806	-18.5	-18.5	0.32 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- B	-161 / 7	0.08 (1)
M- L	-254 / 0	0.27 (1)
L- C	0 / 291	0.07 (1)
C- J	0 / 297	0.05 (1)
J- D	-633 / 0	0.41 (1)
J- E	0 / 569	0.09 (1)
I- E	-26 / 53	0.02 (4)
I- F	0 / 208	0.05 (4)
A- M	0 / 1259	0.28 (1)
F- H	-1541 / 0	0.55 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (1.00")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.42/1.00 (C-D:1) , BC=0.34/1.00 (I-J:4) , WB=0.55/1.00 (F-H:1) , SSI=0.25/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (M) (INPUT = 0.90)
JSI METAL= 0.46 (M) (INPUT = 1.00)

A-18023639

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292613	H7A	1	2	TRUSS DESC.	

Alpa Roof Truss, Maple

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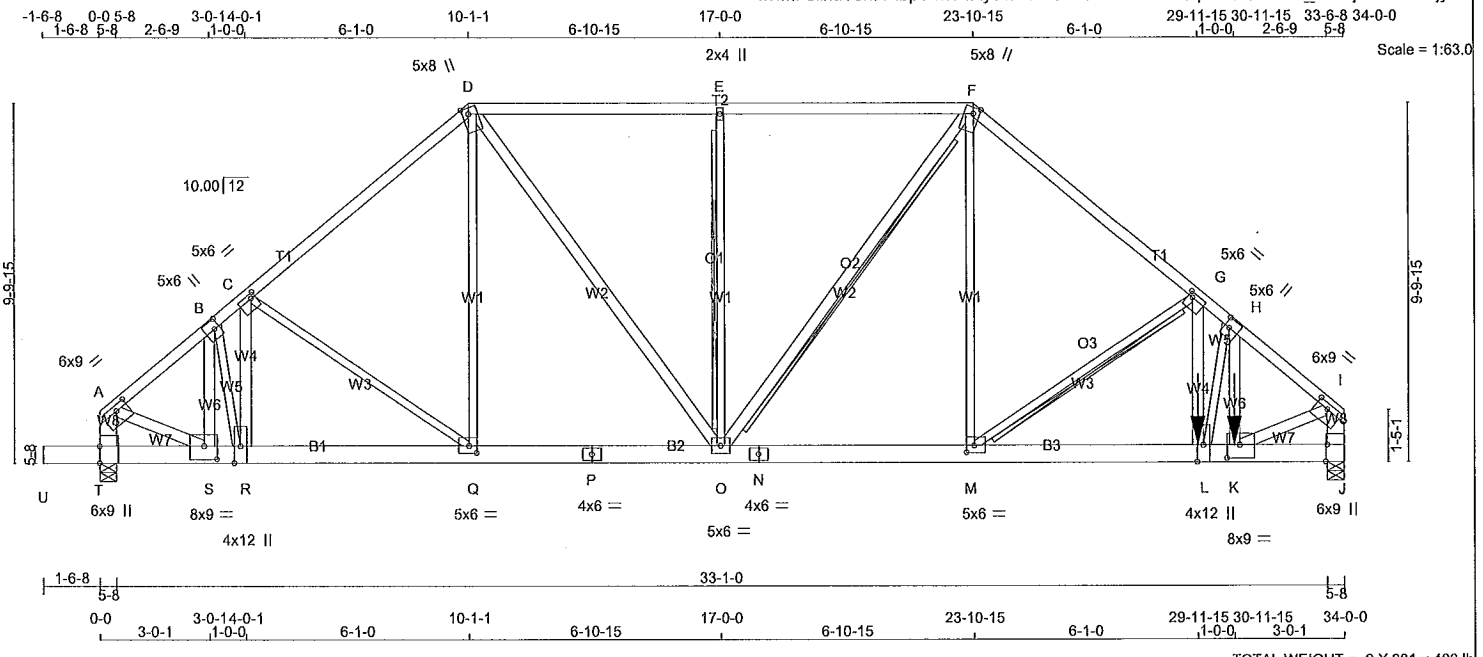


TOTAL WEIGHT = 2 X 201 = 402 lb

[M]

LUMBER

N. L. G. A. RULES

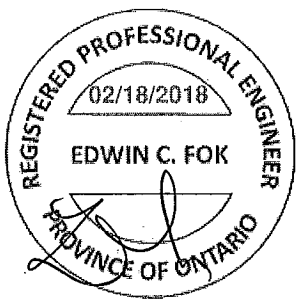
CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
T - A	2x6	DRY	No.2	SPF
J - I	2x6	DRY	No.2	SPF
U - P	2x6	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
N - J	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
B - R	2x3	DRY	No.2	SPF
C - Q	2x3	DRY	No.2	SPF
Q - D	2x3	DRY	No.2	SPF
M - F	2x3	DRY	No.2	SPF
M - G	2x3	DRY	No.2	SPF
L - H	2x3	DRY	No.2	SPF
O - E	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	TOP
D- F	1	12	TOP
F- I	1	12	TOP
T- A	2	12	TOP
J- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
U- P	2	12	TOP
P- N	2	12	TOP
N- J	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
L- H	1	2	SIDE(740.4)
2x3	1	6	
G- L	1	5	SIDE(425.1)
H- K	1	2	SIDE(664.6)
2x4	1	6	

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	2788	0	2788	0	0	5-8	1-8
J	10570	0	10570	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1975	1277 / 0	0 / 0	0 / 0	0 / 0	698 / 0	0 / 0
J	7380	5377 / 0	0 / 0	0 / 0	0 / 0	2004 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, J

BEARING SIZE FACTOR = 1.15 AT JNT(S) J (BASED ON SUPPORT DEPTH = 1-8)

BRACING

FOR SECTION D-F, MAX. PURLIN SPACING = 2.00 FT.

FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.42 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x6 DRY SPF No.2 T-BRACE AT G-M, F-O

2x4 DRY SPF No.2 T-BRACE AT E-O

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-2782 / 0	-78.0	-78.0	0.09 (1)	5.42
B- C	-2822 / 0	-78.0	-78.0	0.24 (1)	5.16
C- D	-3043 / 0	-78.0	-78.0	0.35 (1)	5.00
D- E	-3312 / 0	-85.5	-85.5	0.50 (1)	2.00
E- F	-3312 / 0	-85.5	-85.5	0.50 (1)	2.00
F- G	-4812 / 0	-78.0	-78.0	0.54 (1)	4.05
G- H	-10422 / 0	-78.0	-78.0	0.58 (1)	2.47
H- I	-11168 / 0	-78.0	-78.0	0.64 (1)	2.42
T- A	-2612 / 0	0.0	0.0	0.09 (1)	7.81
J- I	-10160 / 0	0.0	0.0	0.37 (1)	4.73
U- T	0 / 0	-96.5	-96.5	0.04 (1)	10.00
T- S	0 / 0	-18.5	-18.5	0.01 (4)	10.00
S- R	0 / 2183	-18.5	-18.5	0.16 (1)	10.00
R- Q	0 / 2315	-18.5	-18.5	0.18 (1)	10.00
Q- P	0 / 2312	-18.5	-18.5	0.18 (1)	10.00
P- O	0 / 2312	-18.5	-18.5	0.18 (1)	10.00
O- N	0 / 3699	-18.5	-18.5	0.26 (1)	10.00
N- M	0 / 3699	-18.5	-18.5	0.26 (1)	10.00
M- L	0 / 7974	-18.5	-18.5	0.57 (1)	10.00
L- K	0 / 8544	-18.5	-18.5	0.90 (1)	10.00
K- J	0 / 0	-63.7	-63.7	0.35 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- B	-638 / 0	0.05 (1)
B- R	0 / 634	0.08 (1)
R- C	-615 / 0	0.07 (1)
C- Q	-12 / 0	0.01 (4)
Q- D	0 / 151	0.03 (4)
M- F	0 / 3177	0.39 (1)
M- G	-5265 / 0	0.96 (1)
L- G	0 / 5990	0.53 (1)
L- H	-1252 / 0	0.16 (1)
K- H	0 / 1903	0.17 (1)
A- S	0 / 2262	0.20 (1)
K- I	0 / 9008	0.80 (1)
O- F	-655 / 0	0.14 (1)
D- O	0 / 1693	0.15 (1)
O- E	-713 / 0	0.34 (1)

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

GIRDER TYPE: CStdGirder
START DISTANCE = 30-11-14
START SPAN CARRIED = 3-10-8
END DISTANCE = 34-0-0
END SPAN CARRIED = 3-10-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.13")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.13")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.64/1.00 (H-I:1) , BC=0.90/1.00 (K-L:1) , WB=0.96/1.00 (G-M:1) , SSI=0.35/1.00 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

A-18023640

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292613	H7A	1	2	TRUSS DESC.	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	6.0	9.0	1.75	4.00
B	TMWW+t	MT20	5.0	6.0	3.00	1.75
C	TMWW-t	MT20	5.0	6.0	1.50	1.50
D	TTWW+m	MT20	5.0	8.0	2.00	2.00
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	8.0	2.00	2.00
G	TMWW-t	MT20	5.0	6.0	1.50	1.50
H	TMWW+t	MT20	5.0	6.0	3.00	1.75
I	TMVW-t	MT20	6.0	9.0	1.75	4.00
J	BMV1+t	MT20	6.0	9.0	Edge	0.50
K	BMWW-t	MT20	8.0	9.0	4.25	4.25
L	BMWW+t	MT20	4.0	12.0	Edge	
M	BMWW-t	MT20	5.0	6.0	2.25	2.50
N	BS-t	MT20	4.0	6.0		
O	BMWWW-t	MT20	5.0	6.0		
P	BS-t	MT20	4.0	6.0		
Q	BMWW-t	MT20	5.0	6.0	2.25	2.50
R	BMWW+t	MT20	4.0	12.0	Edge	
S	BMWW-t	MT20	8.0	9.0	4.25	4.25
T	BMV1+t	MT20	6.0	9.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 6001.3 lbs FACTORED DOWN AT 30-11-14, AND 3688.0 lbs FACTORED DOWN AT 29-11-14 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
K	30-11-14	-6001	-6001	---	FRONT	VERT	TOTAL
L	29-11-14	-3688	-3688	---	FRONT	VERT	TOTAL

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (A) (INPUT = 0.90)
JSI METAL= 0.94 (A) (INPUT = 1.00)

A-180236440(2)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292613	H8P	7	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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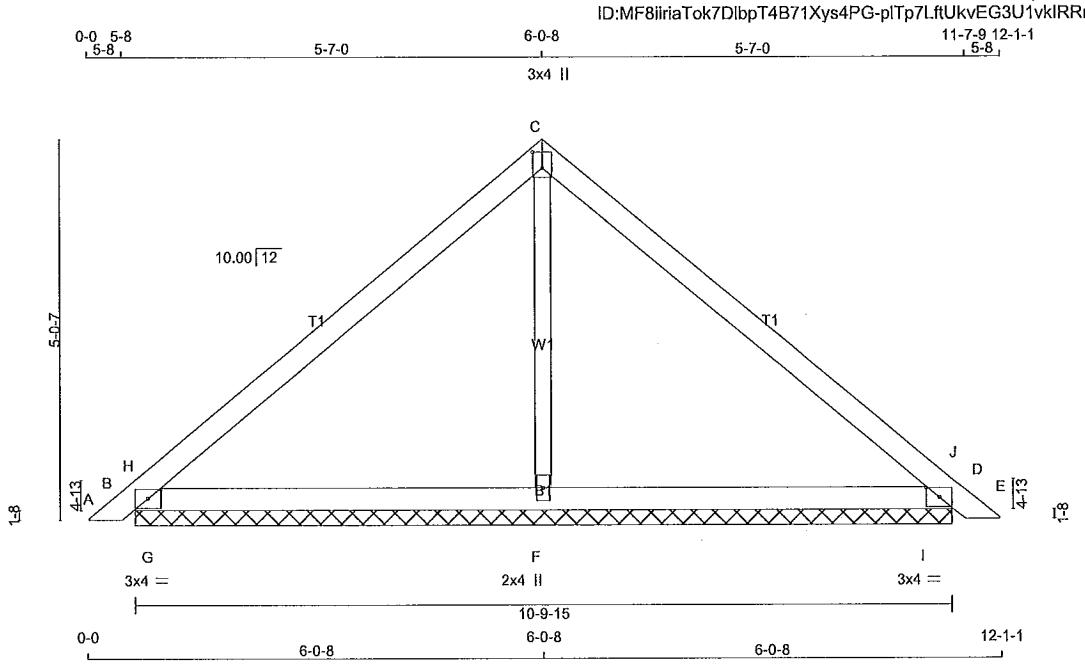


TOTAL WEIGHT = 7 X 34 = 236 lb

(M)

LUMBER
N. L. G. A. RULES

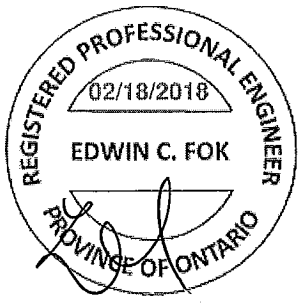
CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTW+p	MT20	3.0	4.0	2.50	1.50
D	TMB1-I	MT20	3.0	4.0	1.50	2.00
F	BMW1+w	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	415	0	415	0	0	10-9-15	1-2-7
D	415	0	415	0	0	10-9-15	1-2-7
F	295	0	295	0	0	10-9-15	1-2-7

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	293	196 / 0	0 / 0	0 / 0	0 / 0	97 / 0	0 / 0
D	293	196 / 0	0 / 0	0 / 0	0 / 0	97 / 0	0 / 0
F	215	106 / 0	0 / 0	0 / 0	0 / 0	109 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 12	-78.0	-78.0	0.02 (1)	10.00	F- C	-75 / 0	0.03 (1)
B- H	-36 / 132	-78.0	-78.0	0.19 (1)	6.25	G- H	-707 / 0	0.00 (1)
H- C	-234 / 0	-78.0	-78.0	0.28 (1)	6.25	I- J	-707 / 0	0.00 (1)
C- J	-234 / 0	-78.0	-78.0	0.28 (1)	6.25			
J- D	-36 / 132	-78.0	-78.0	0.19 (1)	6.25			
D- E	0 / 12	-78.0	-78.0	0.02 (1)	10.00			
B- G	0 / 168	-18.5	-18.5	0.26 (1)	10.00			
G- F	0 / 168	-18.5	-18.5	0.26 (1)	10.00			
F- I	0 / 168	-18.5	-18.5	0.26 (1)	10.00			
I- D	0 / 168	-18.5	-18.5	0.26 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.28/1.00 (C-H:1) , BC=0.26/1.00 (F-G:1) , WB=0.03/1.00 (C-F:1) , SSI=0.54/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.36 (B) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

A-18023641

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292613	H9P	1	2	TRUSS DESC.	

Alpa Roof Truss, Maple

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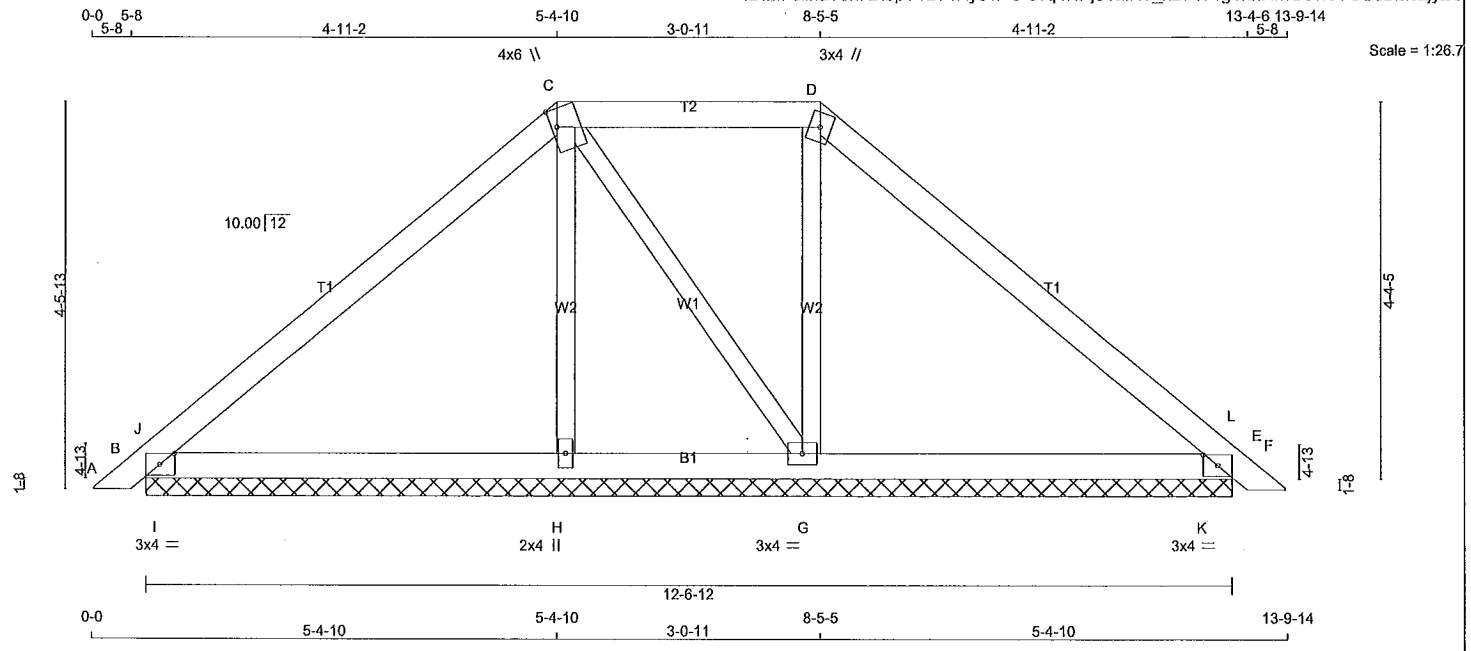


TOTAL WEIGHT = 2 X 44 = 87 lb

(M)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- D	1	12	TOP
D- F	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
B- E	1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

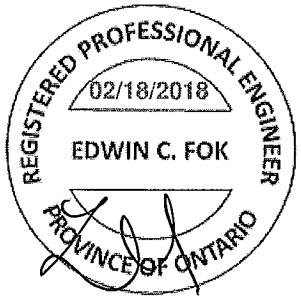
NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTWW+m	MT20	4.0	6.0	2.50	0.75
D	TTW+m	MT20	3.0	4.0		
E	TMB1-I	MT20	3.0	4.0	1.50	2.00
G	BMWW1-I	MT20	3.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	389	0	389	0	0	12-6-12	6-5
E	368	0	368	0	0	12-6-12	6-5
H	222	0	222	0	0	12-6-12	6-5
G	314	0	314	0	0	12-6-12	6-5

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	275	182 / 0	0 / 0	0 / 0	0 / 0	92 / 0	0 / 0
E	260	171 / 0	0 / 0	0 / 0	0 / 0	90 / 0	0 / 0
H	161	83 / 0	0 / 0	0 / 0	0 / 0	78 / 0	0 / 0
G	224	135 / 0	0 / 0	0 / 0	0 / 0	89 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	FORCE (LBS)	MAX CSI (LC)
A- B	0 / 12	-78.0	-78.0	0.01 (1)		10.00	H- C	-113 / 0	0.02 (1)
B- J	-64 / 8	-78.0	-78.0	0.05 (1)		6.25	C- G	-44 / 0	0.01 (1)
J- C	-212 / 0	-78.0	-78.0	0.11 (1)		6.25	G- D	-169 / 0	0.02 (1)
C- D	-125 / 0	-78.0	-78.0	0.06 (1)		6.25	I- J	-483 / 0	0.00 (1)
D- L	-180 / 0	-78.0	-78.0	0.11 (1)		6.25	K- L	-487 / 0	0.00 (1)
L- E	-58 / 43	-78.0	-78.0	0.06 (1)		6.25			
E- F	0 / 12	-78.0	-78.0	0.01 (1)		10.00			
B- I	0 / 154	-18.5	-18.5	0.09 (1)		10.00			
I- H	0 / 154	-18.5	-18.5	0.09 (1)		10.00			
H- G	0 / 151	-18.5	-18.5	0.06 (1)		10.00			
G- K	0 / 130	-18.5	-18.5	0.09 (1)		10.00			
K- E	0 / 130	-18.5	-18.5	0.09 (1)		10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11/1.00 (C-J:1) , BC=0.09/1.00 (H-I:1) , WB=0.02/1.00 (D-G:1) , SSI=0.19/1.00 (E-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.16 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)

A-18023642

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292613	H11A	1	2	TRUSS DESC.	

Alpa Roof Truss, Maple

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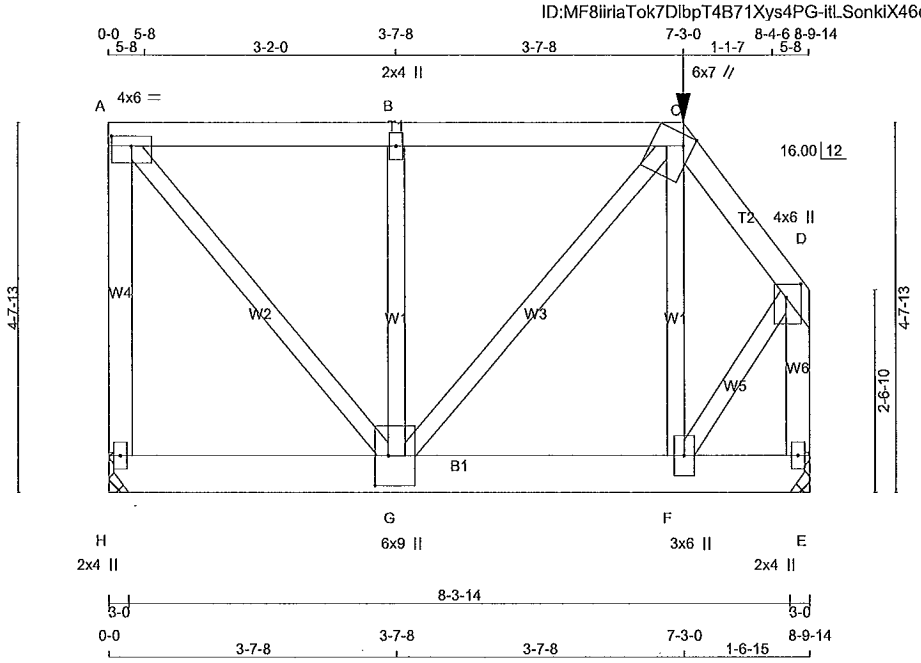


TOTAL WEIGHT = 2 X 51 = 102 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
H - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - E	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
H- A	1	12	TOP
A- C	1	12	SIDE(47.7)
C- D	1	12	SIDE(58.9)
E- D	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
H- E	2	8	SIDE(340.3)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

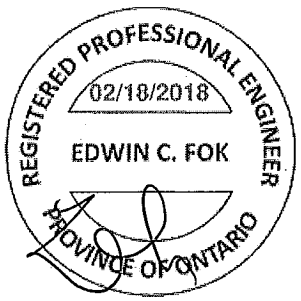
NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H	3593	0	3593	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 2-5	
E	3577	0	3577	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 2-5	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	2562	1564 / 0	0 / 0	0 / 0	0 / 0	998 / 0	0 / 0
E	2551	1555 / 0	0 / 0	0 / 0	0 / 0	995 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.07 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- A	-2548 / 0	0.0	0.0	0.42 (1)	7.10	A- G	0 / 3118	0.39 (1)
A- B	-2016 / 0	-114.5	-114.5	0.12 (1)	6.07	G- B	-464 / 0	0.07 (1)
B- C	-2016 / 0	-114.5	-114.5	0.12 (1)	6.07	G- C	0 / 1203	0.15 (1)
C- D	-2067 / 0	-78.0	-78.0	0.03 (1)	6.13	F- C	0 / 447	0.06 (1)
E- D	-3240 / 0	0.0	0.0	0.23 (1)	6.45	F- D	0 / 1944	0.24 (1)
H- G	0 / 0	-699.2	-699.2	0.30 (1)	10.00			
G- F	0 / 1233	-699.2	-699.2	0.38 (1)	10.00			
F- E	0 / 0	-699.2	-699.2	0.15 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	7-3-0	-49	-49	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 0-0
RIGHT SETBACK = 1-6-15
END SETBACK = 3-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 29-10-6
END DISTANCE = 8-9-14
END SPAN CARRIED = 29-10-6
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.29")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.29")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.42/1.00 (A-H:1) , BC=0.38/1.00 (F-G:1) , WB=0.39/1.00 (A-G:1) , SSI=0.49/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

A-18023643

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292613	H11A	1	2	TRUSS DESC.	

Alpa Roof Truss, Maple

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0	1.50	3.00
B	TMW+w	MT20	2.0	4.0		
C	TTWW+m	MT20	6.0	7.0	Edge	1.50
D	TMVW+p	MT20	4.0	6.0	2.00	2.25
E	BMV1+p	MT20	2.0	4.0		
F	BMWW+t	MT20	3.0	6.0		
G	BMWWW+t	MT20	6.0	9.0	4.50	2.00
H	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 48.7 lbs FACTORED DOWN AT 7-3-0 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (D) (INPUT = 0.90)
JSI METAL= 0.37 (F) (INPUT = 1.00)

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

REGISTERED PROFESSIONAL ENGINEER
02/18/2018
EDWIN C. FOK
PROVINCE OF ONTARIO

A·18023643(2)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292613	H12A	1	2	TRUSS DESC.	

Alpa Roof Truss, Maple

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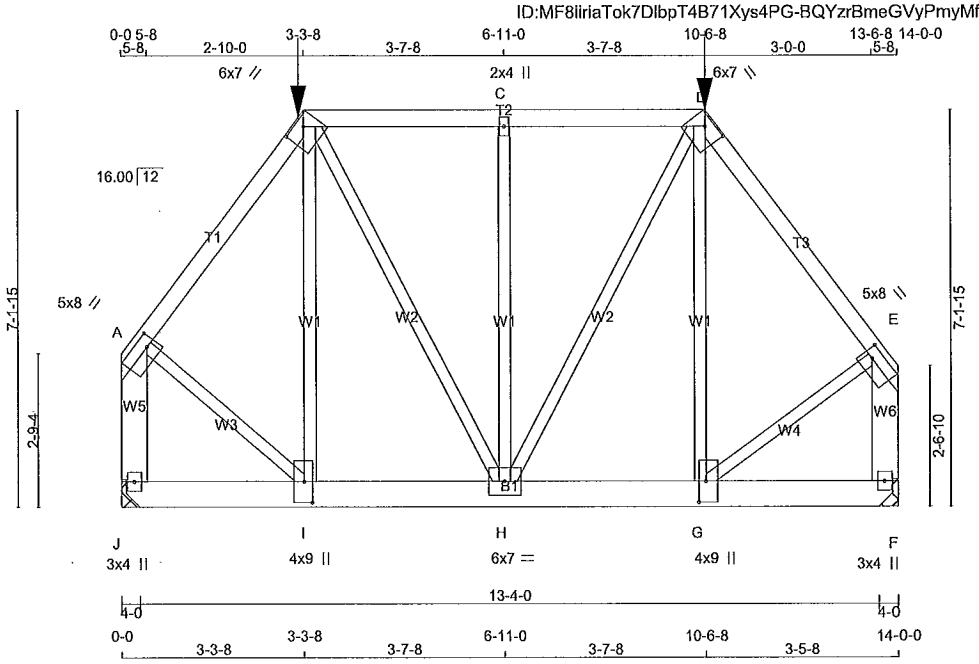


TOTAL WEIGHT = 2 X 89 = 178 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
J - A	2x6	DRY	No.2	SPF
F - E	2x6	DRY	No.2	SPF
J - F	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- B	1	12	SIDE(61.0)
B- D	1	12	SIDE(47.7)
D- E	1	12	SIDE(58.9)
J- A	2	12	TOP
F- E	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
J- F	2	6	SIDE(352.4)
WEBS : (0.122"X3") SPIRAL NAILS			
I- B	1	6	SIDE(8.3)
2x3	1	6	

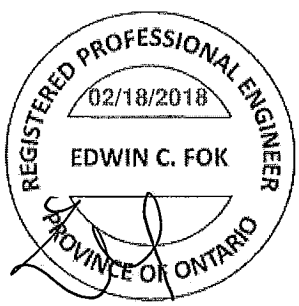
NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
J	5847	0	5847	0	0	IN-SX	IN-SX
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 4-0	
F	5844	0	5844	0	0	HANGER BY OTHERS	
						MIN. SEAT SIZE: 4-0	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	4169	2543 / 0	0 / 0	0 / 0	0 / 0	1627 / 0	0 / 0
F	4167	2541 / 0	0 / 0	0 / 0	0 / 0	1626 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.61 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-3916 / 0	-78.0	-78.0	0.16 (1)	4.68	I- B	0 / 1204	0.15 (1)
B- C	-3158 / 0	-114.5	-114.5	0.14 (1)	5.09	B- H	0 / 1703	0.21 (1)
C- D	-3158 / 0	-114.5	-114.5	0.14 (1)	5.09	H- C	-408 / 0	0.17 (1)
D- E	-4046 / 0	-78.0	-78.0	0.18 (1)	4.61	H- D	0 / 1530	0.19 (1)
J- A	-4853 / 0	0.0	0.0	0.25 (1)	6.58	G- D	0 / 1449	0.18 (1)
F- E	-4790 / 0	0.0	0.0	0.23 (1)	6.62	A- I	0 / 2818	0.35 (1)
						G- E	0 / 2791	0.35 (1)
J- I	0 / 0	-723.3	-723.3	0.29 (1)	10.00			
I- H	0 / 2351	-723.3	-723.3	0.45 (1)	10.00			
H- G	0 / 2433	-723.3	-723.3	0.47 (1)	10.00			
G- F	0 / 0	-723.3	-723.3	0.31 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
B	3-3-8	-102	-102	---	FRONT	VERT	TOTAL
D	10-6-8	-107	-107	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 3-3-8
RIGHT SETBACK = 3-5-8
END SETBACK = 3-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 30-10-6
END DISTANCE = 14-0-0
END SPAN CARRIED = 30-10-6
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.47")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.47")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.25/1.00 (A-J:1) , BC=0.47/1.00 (G-H:1) , WB=0.35/1.00 (A-I:1) , SSI=0.50/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

A-18023644

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292613	H12A	1	2	TRUSS DESC.	

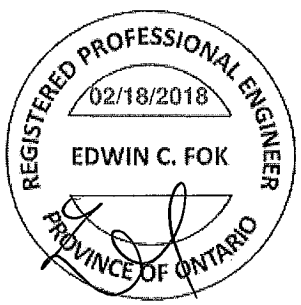
PLATES (table Is In Inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	8.0	2.25	2.00
B	TTWW-h	MT20	6.0	7.0	1.75	4.00
C	TMW+w	MT20	2.0	4.0		
D	TTWW-h	MT20	6.0	7.0	1.75	4.00
E	TMVW-t	MT20	5.0	8.0	2.25	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWW+t	MT20	4.0	9.0	4.50	1.50
H	BMWWW-t	MT20	6.0	7.0		
I	BMWW+t	MT20	4.0	9.0	4.50	1.75
J	BMV1+p	MT20	3.0	4.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 106.7 lbs FACTORED DOWN AT 10-6-8, AND 101.6 lbs FACTORED DOWN AT 3-3-8 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (I) (INPUT = 0.90)
JSI METAL= 0.55 (G) (INPUT = 1.00)

A-180236446

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292613	H13S	3	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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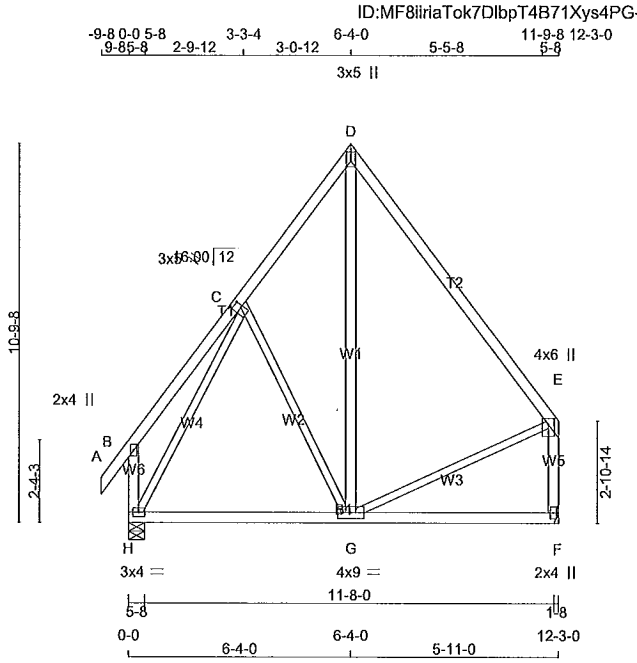


TOTAL WEIGHT = 3 X 72 = 215 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	1650F 1.5E	SPF
H - B	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
G - D	2x4	DRY	No.2	SPF

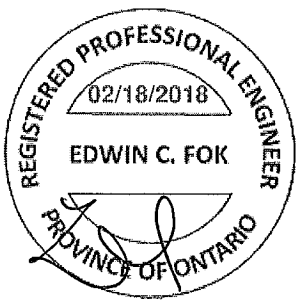
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWW+t	MT20	3.0	5.0	2.25	0.75
D	TTW+p	MT20	3.0	5.0	2.00	Edge
E	TMVW+p	MT20	4.0	6.0	2.00	2.00
F	BMV1+p	MT20	2.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BMVW1-t	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	591	0	591	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
H	662	0	662	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	421	257 / 0	0 / 0	0 / 0	0 / 0	164 / 0	0 / 0
H	470	295 / 0	0 / 0	0 / 0	0 / 0	175 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 28	-78.0	-78.0	0.05 (1)	10.00	C- G	-105 / 0	0.10 (1)
B- C	0 / 24	-78.0	-78.0	0.13 (1)	10.00	G- D	0 / 156	0.03 (4)
C- D	-325 / 0	-78.0	-78.0	0.10 (1)	6.25	H- C	-509 / 0	0.45 (1)
D- E	-303 / 0	-78.0	-78.0	0.30 (1)	6.25	G- E	0 / 197	0.04 (1)
H- B	-162 / 0	0.0	0.0	0.02 (1)	7.81			
F- E	-550 / 0	0.0	0.0	0.09 (1)	7.81			
H- G	0 / 231	-18.5	-18.5	0.22 (4)	10.00			
G- F	0 / 0	-18.5	-18.5	0.20 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	21.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	34.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.30/1.00 (D-E:1) , BC=0.22/1.00 (G-H:4) , WB=0.45/1.00 (C-H:1) , SSI=0.11/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (C) (INPUT = 0.90)
JSI METAL= 0.32 (C) (INPUT = 1.00)

A-18023645

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292613	H13T	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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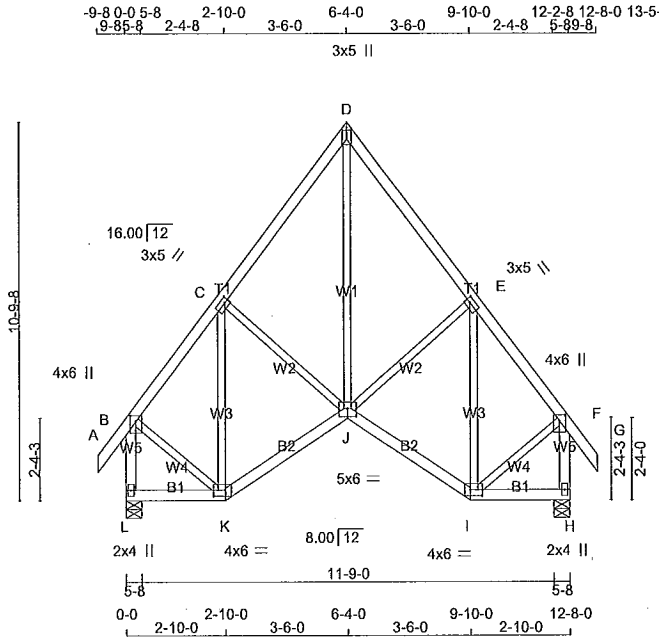


TOTAL WEIGHT = 2 X 76 = 152 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

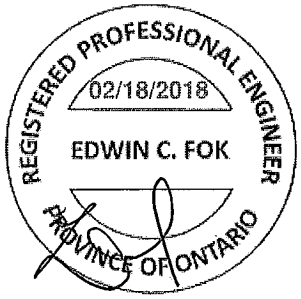
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TMWW-t	MT20	3.0	5.0	1.50	1.50
D	TTW+p	MT20	3.0	5.0	2.00	Edge
E	TMWW-t	MT20	3.0	5.0	1.50	1.50
F	TMVW+p	MT20	4.0	6.0	2.00	2.00
H	BMV1+p	MT20	2.0	4.0		
I	BBWW-t	MT20	4.0	6.0	2.00	4.00
J	BBWWW-p	MT20	5.0	6.0	2.75	3.00
K	BBWW-t	MT20	4.0	6.0	2.00	4.00
L	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
L	682	0	682	0	0	5-8	1-8
H	682	0	682	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	485	304 / 0	0 / 0	0 / 0	0 / 0	181 / 0	0 / 0
H	485	304 / 0	0 / 0	0 / 0	0 / 0	181 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 28	-78.0	-78.0	0.05 (1)	10.00	J- D	0 / 415	0.09 (1)
B- C	-396 / 0	-78.0	-78.0	0.10 (1)	6.25	J- E	-12 / 0	0.00 (1)
C- D	-423 / 0	-78.0	-78.0	0.12 (1)	6.25	I- E	-283 / 0	0.15 (1)
D- E	-423 / 0	-78.0	-78.0	0.12 (1)	6.25	C- J	-12 / 0	0.00 (1)
E- F	-396 / 0	-78.0	-78.0	0.10 (1)	6.25	K- C	-283 / 0	0.15 (1)
F- G	0 / 28	-78.0	-78.0	0.05 (1)	10.00	B- K	0 / 309	0.07 (1)
L- B	-656 / 0	0.0	0.0	0.08 (1)	7.81	I- F	0 / 309	0.07 (1)
H- F	-656 / 0	0.0	0.0	0.08 (1)	7.81			
L- K	0 / 0	-18.5	-18.5	0.04 (4)	10.00			
K- J	0 / 300	-18.5	-18.5	0.09 (4)	10.00			
J- I	0 / 300	-18.5	-18.5	0.09 (4)	10.00			
I- H	0 / 0	-18.5	-18.5	0.04 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.42")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.42")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.12/1.00 (C-D:1) , BC=0.09/1.00 (I-J:4) , WB=0.15/1.00 (E-I:1) , SSI=0.08/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.64 (D) (INPUT = 0.90)
JSI METAL= 0.17 (F) (INPUT = 1.00)

A-18023646

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292613	H14S	3	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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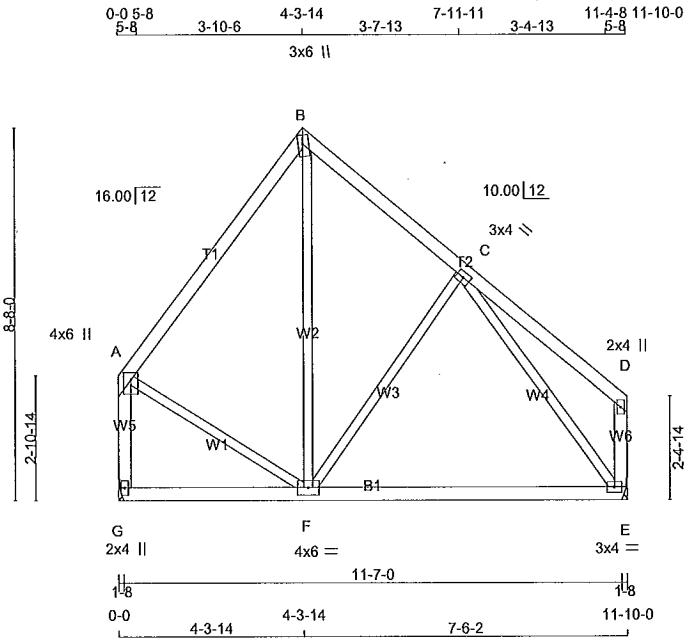


TOTAL WEIGHT = 3 X 59 = 177 lb

[M][F]

LUMBER
N. L. G. A. RULES

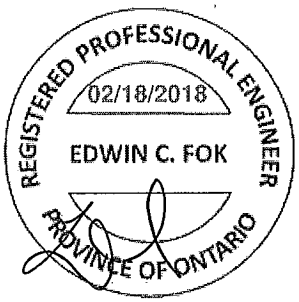
CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
G - A	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is In inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.00	2.00
B	TTW+m	MT20	3.0	6.0	2.50	1.25
C	TMWW-t	MT20	3.0	4.0		
D	TMV+p	MT20	2.0	4.0		
E	BMVW1-t	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
G	571	0	571	0	0	IN-SX	IN-SX
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	
E	571	0	571	0	0	HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	407	248 / 0	0 / 0	0 / 0	0 / 0	159 / 0	0 / 0
E	407	248 / 0	0 / 0	0 / 0	0 / 0	159 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-335 / 0	-78.0	-78.0	0.19 (1)	6.25	F- B	0 / 156	0.04 (4)
B- C	-285 / 0	-78.0	-78.0	0.14 (1)	6.25	F- C	-156 / 0	0.11 (1)
C- D	0 / 22	-78.0	-78.0	0.18 (1)	10.00	A- F	0 / 230	0.05 (1)
G- A	-551 / 0	0.0	0.0	0.09 (1)	7.81	C- E	-496 / 0	0.35 (1)
E- D	-113 / 0	0.0	0.0	0.01 (1)	7.81			
G- F	0 / 0	-18.5	-18.5	0.22 (4)	10.00			
F- E	0 / 291	-18.5	-18.5	0.25 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.39")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.39")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.19/1.00 (A-B:1) , BC=0.25/1.00 (E-F:4) , WB=0.35/1.00 (C-E:1) , SSI=0.11/1.00 (E-F:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (C) (INPUT = 0.90)
JSI METAL= 0.19 (C) (INPUT = 1.00)

A-18023647

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292613	H15A	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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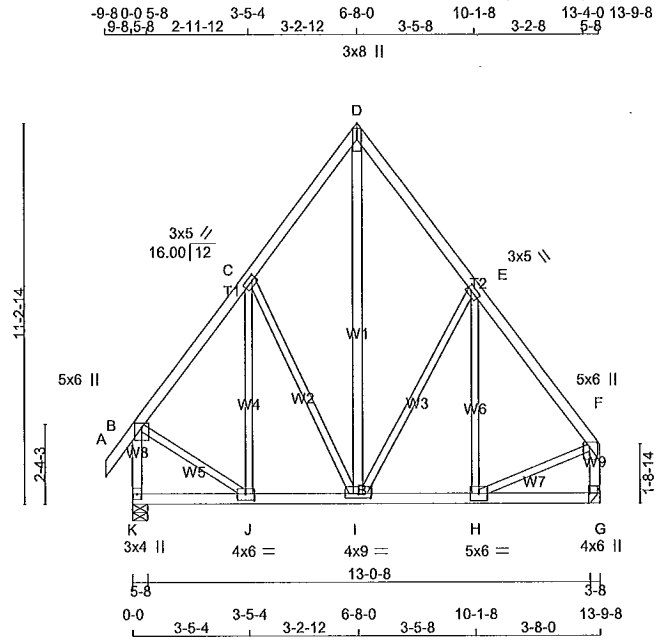


TOTAL WEIGHT = 87 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
I - D	2x4	DRY	No.2	SPF

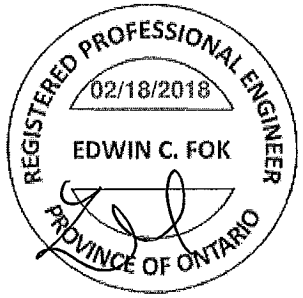
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	1.50	2.50
C	TMWW-t	MT20	3.0	5.0	1.50	1.50
D	TTW+p	MT20	3.0	8.0	3.75	Edge
E	TMWW-t	MT20	3.0	5.0	1.50	1.50
F	TMVW+p	MT20	5.0	6.0	1.75	2.75
G	BMV1+t	MT20	4.0	6.0	Edge	0.50
H	BMWW-t	MT20	5.0	6.0		
I	BMWWW-t	MT20	4.0	9.0	1.50	4.50
J	BMWW-t	MT20	4.0	6.0	2.25	3.00
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
K	2441	0	2441	0	0	5-8	4-7
G	2370	0	2370	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	1739	1069 / 0	0 / 0	0 / 0	0 / 0	669 / 0	0 / 0
G	1690	1031 / 0	0 / 0	0 / 0	0 / 0	658 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 28	-78.0	-78.0	0.05 (1)	10.00	J- C	0 / 430	0.11 (1)
B- C	-1634 / 0	-78.0	-78.0	0.20 (1)	4.99	C- I	-499 / 0	0.53 (1)
C- D	-1308 / 0	-78.0	-78.0	0.17 (1)	5.46	I- D	0 / 1853	0.33 (1)
D- E	-1309 / 0	-78.0	-78.0	0.20 (1)	5.41	I- E	-664 / 0	0.67 (1)
E- F	-1800 / 0	-78.0	-78.0	0.24 (1)	4.75	H- E	0 / 675	0.17 (1)
K- B	-2069 / 0	0.0	0.0	0.28 (1)	5.83	B- J	0 / 1143	0.28 (1)
G- F	-1976 / 0	0.0	0.0	0.23 (1)	5.95	H- F	0 / 1165	0.29 (1)
K- J	0 / 0	-265.7	-265.7	0.44 (1)	10.00			
J- I	0 / 993	-265.7	-265.7	0.61 (1)	10.00			
I- H	0 / 1094	-265.7	-265.7	0.70 (1)	10.00			
H- G	0 / 0	-265.7	-265.7	0.51 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 12-3-0
END DISTANCE = 13-9-8
END SPAN CARRIED = 12-3-0
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.46")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.46")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.28/1.00 (B-K:1) , BC=0.70/1.00 (H-I:1) , WB=0.67/1.00 (E-I:1) , SSI=0.50/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (E) (INPUT = 0.90)
JSI METAL= 0.55 (B) (INPUT = 1.00)

A-18023648

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292613	H15T	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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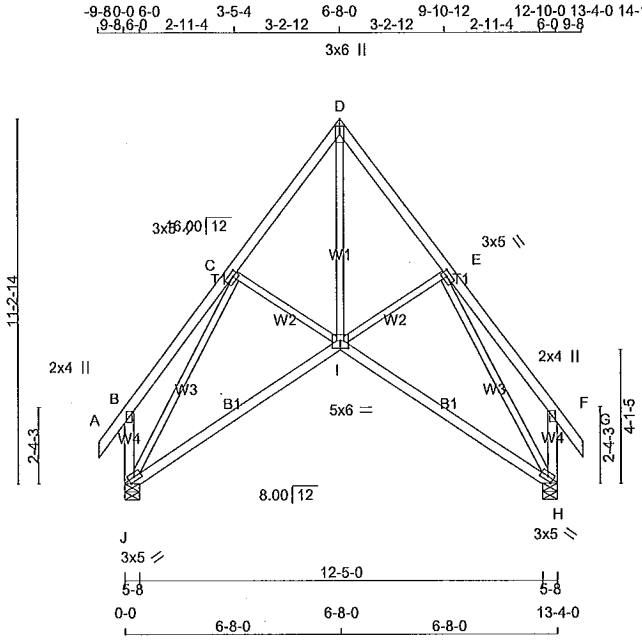


TOTAL WEIGHT = 2 X 75 = 150 lb

[M][F]

LUMBER
N. L. G. A. RULES

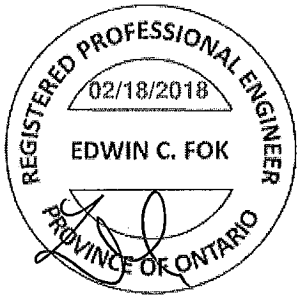
CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWW-t	MT20	3.0	5.0	1.50	2.00
D	TTW+p	MT20	3.0	6.0		
E	TMWW-t	MT20	3.0	5.0	1.50	2.00
F	TMV+p	MT20	2.0	4.0		
H	BMVW1-t	MT20	3.0	5.0	1.50	2.25
I	BBWWW-p	MT20	5.0	6.0	2.75	3.00
J	BMVW1-t	MT20	3.0	5.0	1.50	2.25

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	714	0	714	0	0	5-8	1-8
H	714	0	714	0	0	5-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	508	318 / 0	0 / 0	0 / 0	0 / 0	190 / 0	0 / 0
H	508	318 / 0	0 / 0	0 / 0	0 / 0	190 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 28	-78.0	-78.0	0.05 (1)	10.00	I- D	0 / 646	0.15 (1)
B- C	0 / 23	-78.0	-78.0	0.13 (1)	10.00	I- E	-43 / 8	0.01 (1)
C- D	-555 / 0	-78.0	-78.0	0.10 (1)	6.25	C- I	-43 / 8	0.01 (1)
D- E	-555 / 0	-78.0	-78.0	0.10 (1)	6.25	J- C	-802 / 0	0.80 (1)
E- F	0 / 23	-78.0	-78.0	0.13 (1)	10.00	E- H	-802 / 0	0.80 (1)
F- G	0 / 28	-78.0	-78.0	0.05 (1)	10.00			
J- B	-171 / 0	0.0	0.0	0.02 (1)	7.81			
H- F	-171 / 0	0.0	0.0	0.02 (1)	7.81			
J- I	0 / 428	-18.5	-18.5	0.28 (4)	10.00			
I- H	0 / 428	-18.5	-18.5	0.28 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.44")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.44")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.13/1.00 (E-F:1) , BC=0.28/1.00 (H-I:4) , WB=0.80/1.00 (C-J:1) , SSI=0.07/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (C) (INPUT = 0.90)
JSI METAL= 0.24 (E) (INPUT = 1.00)

A-18023649

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292613	H16A	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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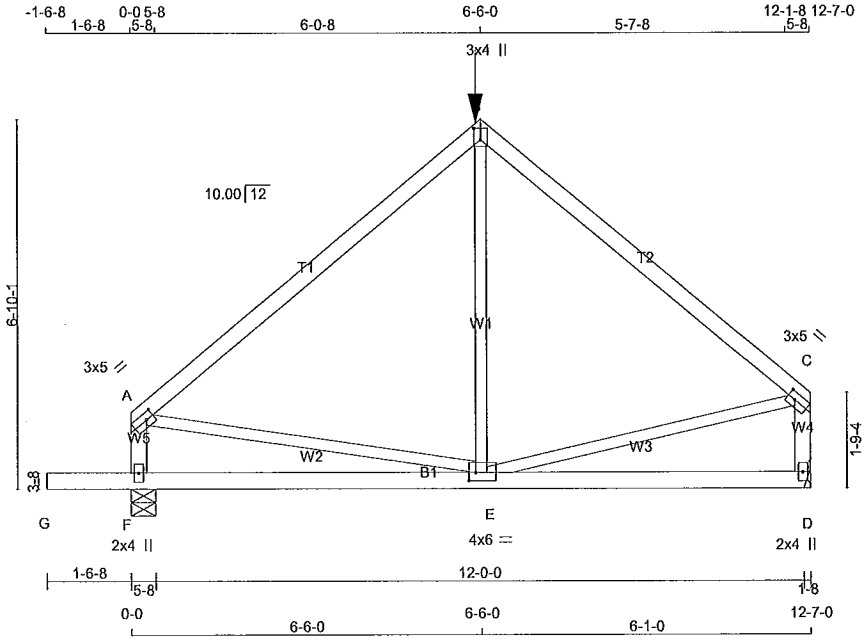


TOTAL WEIGHT = 53 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	2100F 1.8E	SPF
B - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
G - D	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

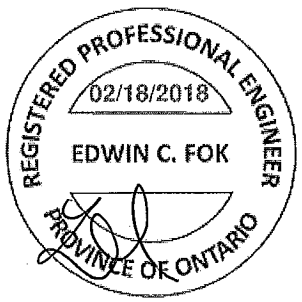
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	3.0	5.0	1.50	1.75
B	TTW+p	MT20	3.0	4.0	2.50	1.50
C	TMVW-t	MT20	3.0	5.0	1.50	1.75
D	BMV1+p	MT20	2.0	4.0		
E	BMWWW-t	MT20	4.0	6.0	1.75	1.50
F	BMV1+p	MT20	2.0	4.0		

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 388.4 lbs FACTORED DOWN AT 6-6-0 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
D	862	0	862	0	0	IN-SX	IN-SX
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	
F	998	0	998	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	615	372 / 0	0 / 0	0 / 0	0 / 0	243 / 0	0 / 0
F	712	430 / 0	0 / 0	0 / 0	0 / 0	282 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.10 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-685 / 0	-78.0	-78.0	0.44	(1)	6.25	E- B	-1 / 155	0.06 (4)
B- C	-685 / 0	-78.0	-78.0	0.60	(1)	6.10	A- E	0 / 533	0.13 (1)
F- A	-777 / 0	0.0	0.0	0.09	(1)	7.81	E- C	0 / 542	0.13 (1)
D- C	-805 / 0	0.0	0.0	0.10	(1)	7.81			
G- F	0 / 0	-96.5	-96.5	0.17	(1)	10.00			
F- E	0 / 0	-27.2	-27.2	0.34	(4)	10.00			
E- D	0 / 0	-27.2	-27.2	0.34	(4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
B	6-6-0	-388	-388	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 6-6-0
RIGHT SETBACK = 6-1-0
END SETBACK = 3-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.42")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.42")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.60/1.00 (B-C:1) , BC=0.34/1.00 (E-F:4) , WB=0.13/1.00 (C-E:1) , SSI=0.17/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

A-18023650

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292613	H17	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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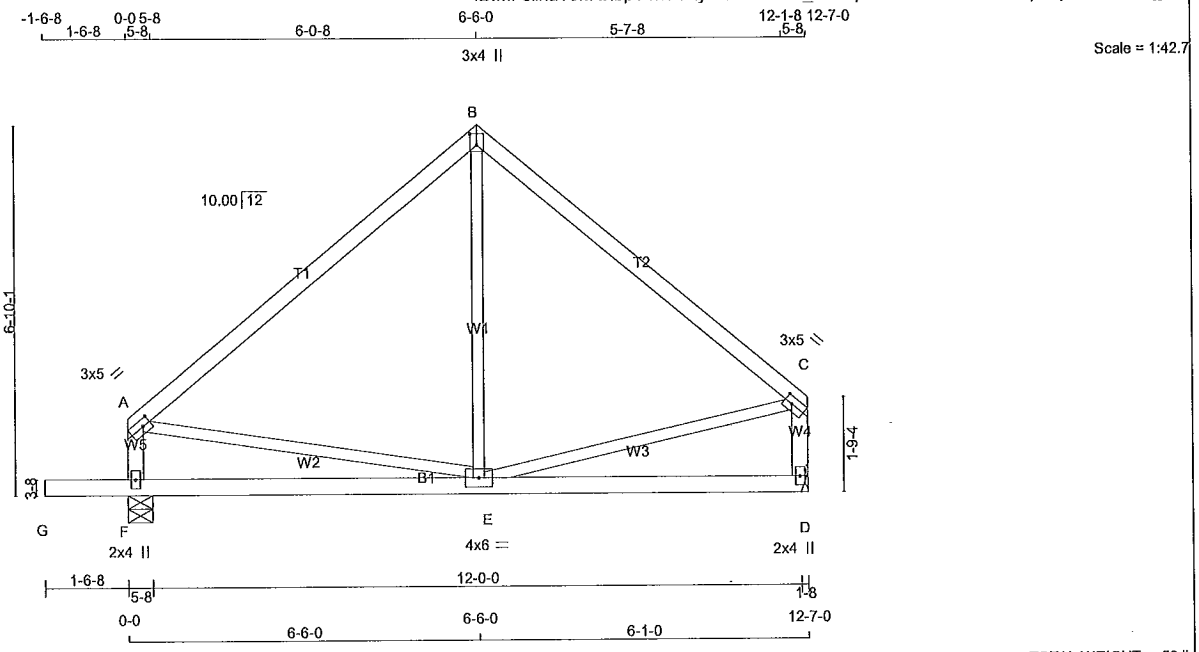


TOTAL WEIGHT = 53 lb

[M][F]

LUMBER

N. L. G. A. RULES

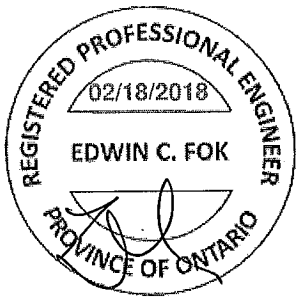
CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	2100F 1.8E	SPF
B - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
G - D	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	3.0	5.0	1.50	1.75
B	TTW+p	MT20	3.0	4.0	2.50	1.50
C	TMVW-t	MT20	3.0	5.0	1.50	1.75
D	BMV1+p	MT20	2.0	4.0		
E	BMWWW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
D	607	0	607	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
F	756	0	756	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	433	264 / 0	0 / 0	0 / 0	0 / 0	169 / 0	0 / 0
F	539	329 / 0	0 / 0	0 / 0	0 / 0	210 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-399 / 0	-78.0	-78.0	0.28 (1)	6.25	E- B	0 / 110	0.04 (4)
B- C	-399 / 0	-78.0	-78.0	0.37 (1)	6.25	A- E	0 / 310	0.07 (1)
F- A	-558 / 0	0.0	0.0	0.06 (1)	7.81	E- C	0 / 316	0.07 (1)
D- C	-568 / 0	0.0	0.0	0.06 (1)	7.81			
G- F	0 / 0	-96.5	-96.5	0.16 (1)	10.00			
F- E	0 / 0	-18.5	-18.5	0.21 (4)	10.00			
E- D	0 / 0	-18.5	-18.5	0.21 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.42")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.42")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.37/1.00 (B-C:1) , BC=0.21/1.00 (D-E:4) , WB=0.07/1.00 (C-E:1) , SSI=0.15/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.57 (E) (INPUT = 0.90)
JSI METAL= 0.17 (A) (INPUT = 1.00)

A-18023651

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292614	H1T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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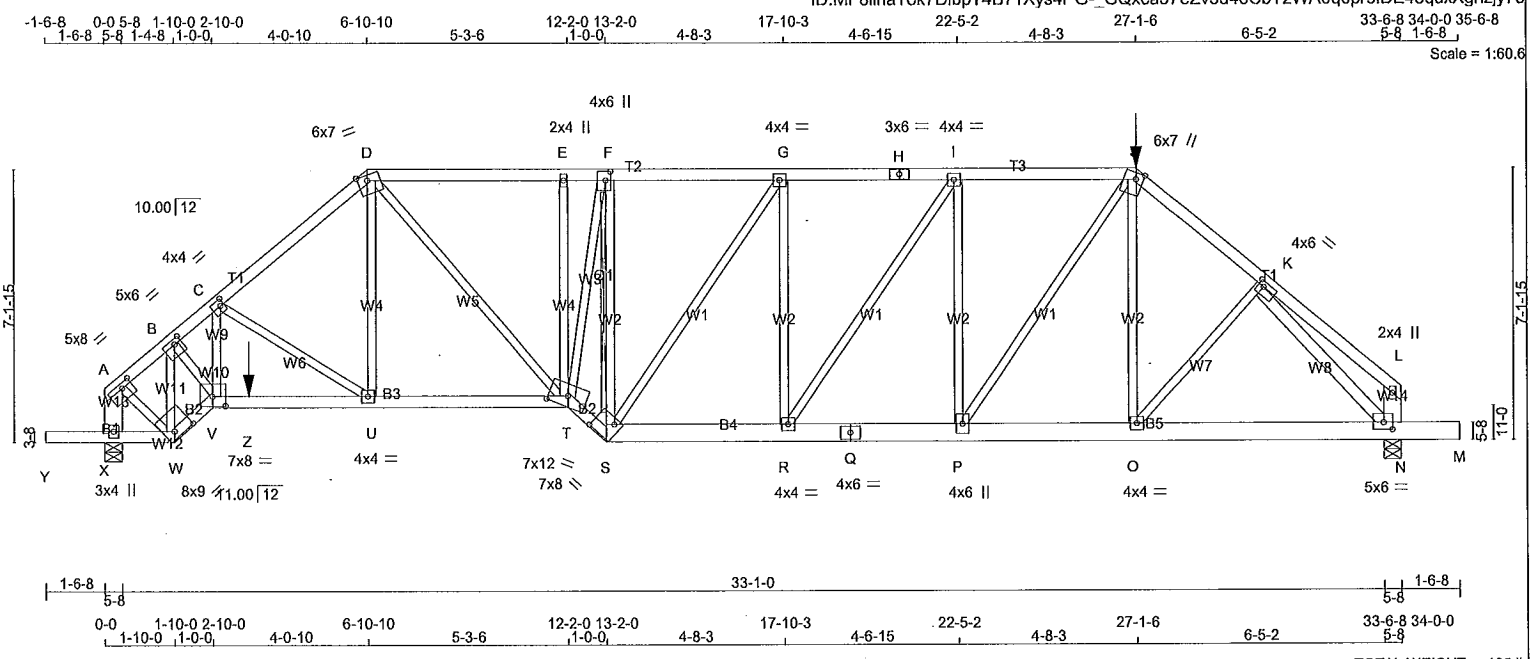


TOTAL WEIGHT = 185 lb [M]

LUMBER

N. L. G. A. RULES

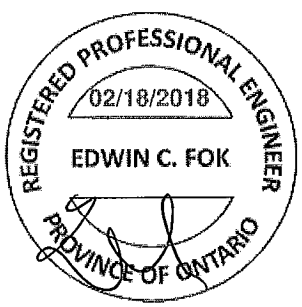
CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
X - A	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
Y - W	2x4	DRY	No.2	SPF
W - V	2x4	DRY	No.2	SPF
V - T	2x4	DRY	No.2	SPF
T - S	2x4	DRY	No.2	SPF
S - Q	2x6	DRY	No.2	SPF
Q - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
K - N	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	8.0	1.50	3.25
B	TMWW-t	MT20	5.0	6.0	1.50	2.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW-m	MT20	6.0	7.0	Edge	3.00
E	TMW+w	MT20	2.0	4.0		
F	TMWW+t	MT20	4.0	6.0	2.75	1.50
G	TMWW-t	MT20	4.0	4.0		
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	4.0	4.0		
J	TTWW+m	MT20	6.0	7.0	2.00	2.25
K	TMWW-t	MT20	4.0	6.0	1.50	1.75
L	TMV+p	MT20	2.0	4.0		
N	BMW1-t	MT20	5.0	6.0	2.25	2.75
O, R, U						
O	BMWW-t	MT20	4.0	4.0		
P	BMWW+t	MT20	4.0	6.0		
Q	BS-t	MT20	4.0	6.0		
S	BBWW-h	MT20	7.0	8.0	5.25	5.75
T	BBWWW-m	MT20	7.0	12.0	3.25	6.00
V	BBWW-t	MT20	7.0	8.0	3.00	4.00

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
X	2646	0	2646	0	0	5-8	2-14
N	2517	0	2517	0	0	5-8	2-11

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
X	1882	1174 / 0	0 / 0	0 / 0	0 / 0	708 / 0	0 / 0
N	1795	1092 / 0	0 / 0	0 / 0	0 / 0	703 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.34 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT F-S

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-2159 / 0	-78.0	-78.0	0.15 (1)	4.47	W- B	-2405 / 0	0.45 (1)
B- C	-3381 / 0	-78.0	-78.0	0.27 (1)	3.54	B- V	0 / 1940	0.48 (1)
C- D	-3087 / 0	-78.0	-78.0	0.36 (1)	3.67	V- C	0 / 89	0.02 (1)
D- E	-3297 / 0	-78.0	-78.0	0.57 (1)	3.34	C- U	-352 / 0	0.14 (1)
E- F	-3293 / 0	-114.5	-114.5	0.40 (1)	3.43	U- D	0 / 608	0.15 (1)
F- G	-2971 / 0	-114.5	-114.5	0.48 (1)	3.57	D- T	0 / 1399	0.35 (1)
G- H	-3125 / 0	-114.5	-114.5	0.58 (1)	3.34	T- E	-244 / 0	0.15 (1)
H- I	-3125 / 0	-114.5	-114.5	0.58 (1)	3.34	T- F	0 / 1951	0.48 (1)
I- J	-2833 / 0	-114.5	-114.5	0.55 (1)	3.54	S- F	-2280 / 0	0.81 (1)
J- K	-2706 / 0	-78.0	-78.0	0.26 (1)	3.97	S- G	-271 / 0	0.38 (1)
K- L	0 / 17	-78.0	-78.0	0.14 (1)	10.00	R- G	-296 / 0	0.25 (1)
X- A	-2418 / 0	0.0	0.0	0.18 (1)	6.59	R- I	0 / 520	0.13 (1)
N- L	-111 / 0	0.0	0.0	0.01 (1)	7.81	P- I	-1011 / 0	0.86 (1)
						P- J	0 / 1360	0.34 (1)
Y- X	0 / 0	-96.5	-96.5	0.17 (1)	10.00	O- J	0 / 139	0.05 (4)
X- W	0 / 0	-18.5	-18.5	0.17 (1)	10.00	O- K	0 / 169	0.04 (1)
W- V	0 / 2095	-18.5	-18.5	0.37 (1)	10.00	A- W	0 / 1900	0.47 (1)
V- Z	0 / 2650	-18.5	-18.5	0.80 (1)	10.00	K- N	-2921 / 0	0.90 (1)
Z- U	0 / 2650	-63.7	-63.7	0.80 (1)	10.00			
U- T	0 / 2365	-63.7	-63.7	0.75 (1)	10.00			
T- S	0 / 3901	-27.2	-27.2	0.69 (1)	10.00			
S- R	0 / 3125	-27.2	-27.2	0.46 (1)	10.00			
R- Q	0 / 2834	-27.2	-27.2	0.41 (1)	10.00			
Q- P	0 / 2834	-27.2	-27.2	0.41 (1)	10.00			
P- O	0 / 2060	-27.2	-27.2	0.34 (1)	10.00			
O- N	0 / 1947	-27.2	-27.2	0.34 (1)	10.00			
N- M	0 / 0	-96.5	-96.5	0.08 (1)	10.00			

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CStdGirder
START DISTANCE = 3-9-4
START SPAN CARRIED = 3-10-8
END DISTANCE = 12-2-0
END SPAN CARRIED = 3-10-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 6-10-10
END SETBACK = 3-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 21-10-0 OF SPAN MEASURED FROM THE RIGHT.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.13")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (1.13")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.34")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")

A-18023652

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292614	H2T	5	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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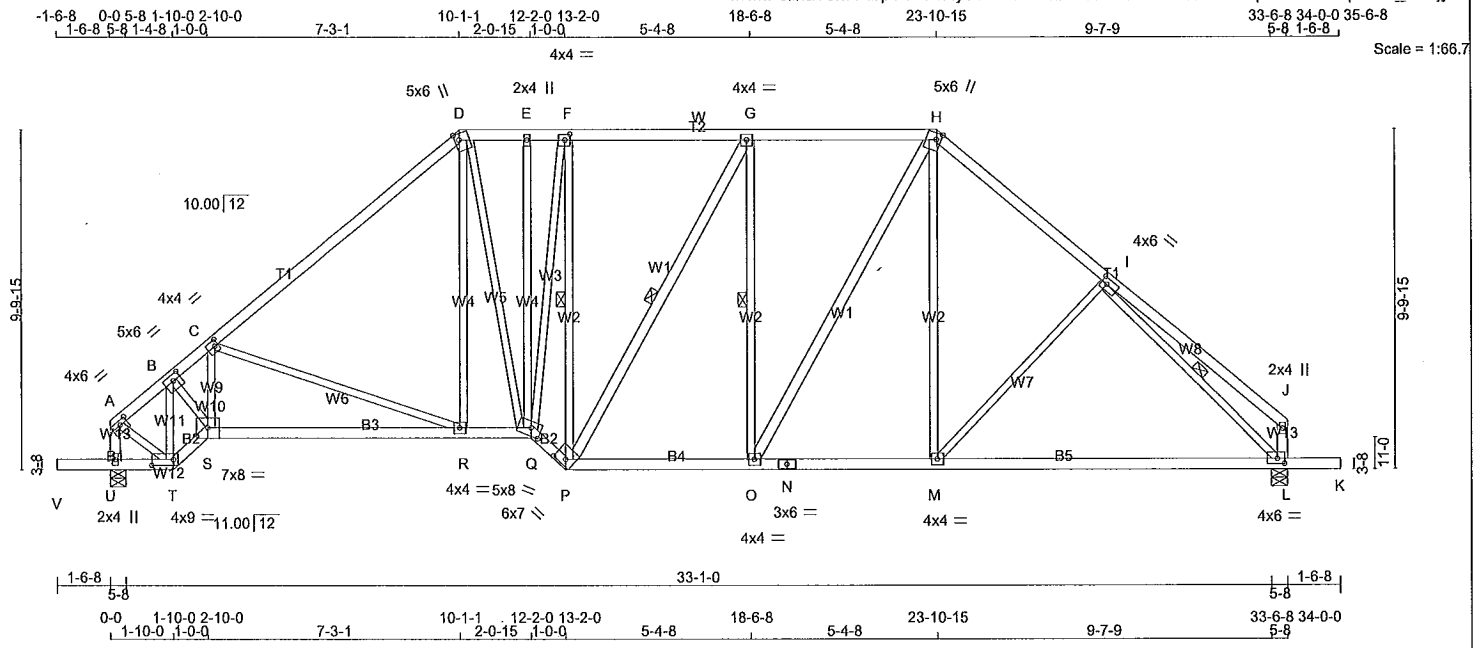


TOTAL WEIGHT = 5 X 190 = 951 lb

[M][F]

LUMBER

N. L. G. A. RULES

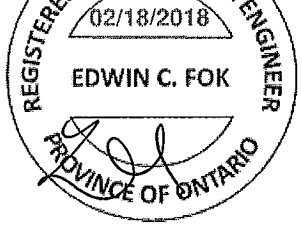
CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
U - A	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
V - T	2x4	DRY	No.2	SPF
T - S	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - P	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
P - G	2x4	DRY	No.2	SPF
O - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0	1.50	2.75
B	TMWW-t	MT20	5.0	6.0	2.00	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0	2.00	1.75
G	TMWW-t	MT20	4.0	4.0		
H	TTWW+m	MT20	5.0	6.0	2.25	1.50
I	TMWW-t	MT20	4.0	6.0	2.00	2.00
J	TMV+p	MT20	2.0	4.0		
L	BMVW1-t	MT20	4.0	6.0	1.75	2.50
M, O, R						
M	BMWW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	6.0		
P	BBWW-h	MT20	6.0	7.0	2.00	4.00
Q	BBWWW-m	MT20	5.0	8.0	2.75	3.50
S	BBWW-I	MT20	7.0	8.0		
T	BBWW-I	MT20	4.0	9.0	2.00	7.50
U	BMV1+p	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
U	1844	0	1844	0	0	IN-SX	IN-SX
L	1837	0	1837	0	0	5-8	2-12
							1-15

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1319	780 / 0	0 / 0	0 / 0	0 / 0	539 / 0	0 / 0
L	1314	777 / 0	0 / 0	0 / 0	0 / 0	537 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

FOR SECTION D-H, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.09 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-P, G-P, G-O, I-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
FR-TO.					
A- B	-1413 / 0	-78.0	-78.0	0.14 (1)	5.35
B- C	-2159 / 0	-78.0	-78.0	0.48 (1)	4.09
C- D	-1879 / 0	-78.0	-78.0	0.66 (1)	4.25
D- E	-1548 / 0	-85.5	-85.5	0.13 (1)	2.00
E- F	-1548 / 0	-85.5	-85.5	0.17 (1)	2.00
F- W	-1469 / 0	-85.5	-85.5	0.31 (1)	2.00
W- G	-1469 / 0	-85.5	-85.5	0.31 (1)	2.00
G- H	-1547 / 0	-85.5	-85.5	0.31 (1)	2.00
H- I	-1712 / 0	-78.0	-78.0	0.30 (1)	4.81
I- J	0 / 29	-78.0	-78.0	0.32 (1)	10.00
U- A	-1616 / 0	0.0	0.0	0.17 (1)	6.51
L- J	-155 / 0	0.0	0.0	0.02 (1)	7.81
V- U	0 / 0	-96.5	-96.5	0.16 (1)	10.00
U- T	0 / 0	-18.5	-18.5	0.12 (1)	10.00
T- S	0 / 1350	-18.5	-18.5	0.22 (1)	10.00
S- R	0 / 1873	-18.5	-18.5	0.43 (1)	10.00
R- Q	0 / 1416	-18.5	-18.5	0.38 (4)	10.00
Q- P	0 / 1976	-18.5	-18.5	0.32 (1)	10.00
P- O	0 / 1547	-18.5	-18.5	0.33 (1)	10.00
O- N	0 / 1293	-18.5	-18.5	0.54 (4)	10.00
N- M	0 / 1293	-18.5	-18.5	0.54 (4)	10.00
M- L	0 / 1393	-18.5	-18.5	0.54 (4)	10.00
L- K	0 / 0	-96.5	-96.5	0.16 (1)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
T- B	-1563 / 0	0.27 (1)
B- S	0 / 1517	0.34 (1)
S- C	-262 / 0	0.05 (1)
C- R	-488 / 0	0.60 (1)
R- D	0 / 290	0.07 (4)
D- Q	0 / 553	0.12 (1)
Q- E	0 / 84	0.02 (1)
Q- F	0 / 717	0.16 (1)
P- F	-1136 / 0	0.74 (1)
P- G	-158 / 0	0.10 (1)
O- G	-383 / 0	0.25 (1)
O- H	0 / 517	0.08 (1)
M- H	0 / 296	0.07 (4)
M- I	-150 / 8	0.16 (1)
A- T	0 / 1225	0.28 (1)
I- L	-2015 / 0	0.73 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.13")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (1.13")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.34")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.66/1.00 (C-D:1) , BC=0.54/1.00 (M-O:4) , WB=0.74/1.00 (F-P:1) , SSI=0.28/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

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CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292614	H2TA	1	2	TRUSS DESC.	

Alpa Roof Truss, Maple

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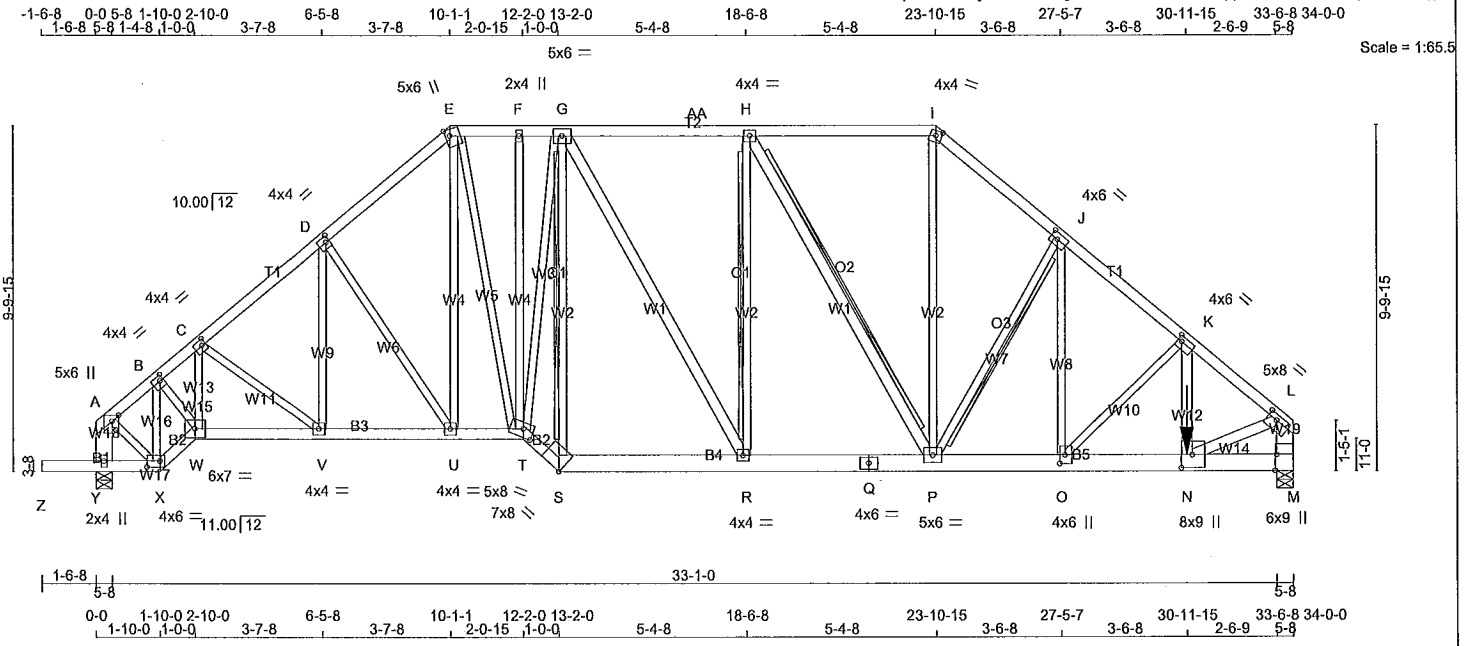


LUMBER

N. L. G. A. RULES

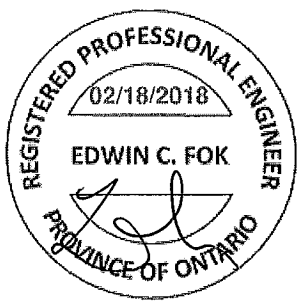
CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
Y - A	2x6	DRY	No.2	SPF
M - L	2x6	DRY	No.2	SPF
Z - X	2x4	DRY	No.2	SPF
X - W	2x4	DRY	No.2	SPF
W - T	2x4	DRY	No.2	SPF
T - S	2x4	DRY	No.2	SPF
S - Q	2x6	DRY	No.2	SPF
Q - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
G - R	2x4	DRY	No.2	SPF
H - P	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- E	1	12	TOP
E- I	1	12	TOP
I- L	1	12	TOP
Y- A	2	12	TOP
M- L	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
Z- X	1	12	TOP
X- W	1	12	TOP
W- T	1	12	TOP
T- S	1	12	TOP
S- Q	2	12	TOP
Q- M	2	12	SIDE(165.4)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
K- N	2	2	SIDE(1405.0)
2x4	1	6	

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
Y	2380	0	2380	0	0	5-8	1-8
M	7289	0	7289	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Y	1692	1062 / 0	0 / 0	0 / 0	0 / 0	630 / 0	0 / 0
M	5101	3650 / 0	0 / 0	0 / 0	0 / 0	1452 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Y, M
BEARING SIZE FACTOR = 1.15 AT JNT(S) M (BASED ON SUPPORT DEPTH = 1-8)

BRACING

FOR SECTION E-I, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.36 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT G-S, H-R, J-P
2x6 DRY SPF No.2 T-BRACE AT H-P

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1917 / 0	-78.0	-78.0	0.05 (1)	6.25
B- C	-3004 / 0	-78.0	-78.0	0.05 (1)	5.30
C- D	-2892 / 0	-78.0	-78.0	0.11 (1)	5.31
D- E	-2674 / 0	-78.0	-78.0	0.11 (1)	5.47
E- F	-2304 / 0	-85.5	-85.5	0.09 (1)	2.00
F- G	-2305 / 0	-85.5	-85.5	0.14 (1)	2.00
G-AA	-2607 / 0	-85.5	-85.5	0.22 (1)	2.00
AA- H	-2607 / 0	-85.5	-85.5	0.22 (1)	2.00
H- I	-2655 / 0	-85.5	-85.5	0.22 (1)	2.00
I- J	-3457 / 0	-78.0	-78.0	0.12 (1)	4.94
J- K	-4775 / 0	-78.0	-78.0	0.17 (1)	4.29
K- L	-7734 / 0	-78.0	-78.0	0.28 (1)	3.36
Y- A	-2152 / 0	0.0	0.0	0.08 (1)	7.81
M- L	-7051 / 0	0.0	0.0	0.25 (1)	5.63
Z- Y	0 / 0	-96.5	-96.5	0.09 (1)	10.00
Y- X	0 / 0	-18.5	-18.5	0.09 (1)	10.00
X- W	0 / 1865	-18.5	-18.5	0.17 (1)	10.00
W- V	0 / 2334	-18.5	-18.5	0.23 (1)	10.00
V- U	0 / 2233	-18.5	-18.5	0.22 (1)	10.00
U- T	0 / 2038	-18.5	-18.5	0.21 (1)	10.00
T- S	0 / 2938	-18.5	-18.5	0.26 (1)	10.00
S- R	0 / 2232	-18.5	-18.5	0.17 (1)	10.00
R- Q	0 / 2607	-18.5	-18.5	0.20 (1)	10.00
Q- P	0 / 2607	-18.5	-18.5	0.20 (1)	10.00
P- O	0 / 3682	-18.5	-18.5	0.27 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
X- B	-2145 / 0	0.20 (1)
B- W	0 / 1673	0.21 (1)
W- C	-54 / 10	0.01 (1)
C- V	-126 / 0	0.02 (1)
V- D	0 / 120	0.02 (4)
D- U	-356 / 0	0.15 (1)
U- E	0 / 347	0.04 (1)
E- T	0 / 1125	0.14 (1)
T- F	0 / 148	0.02 (1)
T- G	0 / 737	0.09 (1)
S- G	-1880 / 0	0.91 (1)
G- R	0 / 770	0.07 (1)
R- H	-588 / 0	0.28 (1)
H- P	-82 / 99	0.02 (4)
P- I	0 / 1905	0.24 (1)
P- J	-2132 / 0	0.57 (1)
O- J	0 / 2382	0.29 (1)
O- K	-3180 / 0	0.65 (1)
N- K	0 / 3893	0.34 (1)
A- X	0 / 1692	0.21 (1)
N- L	0 / 6263	0.55 (1)

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

GIRDER TYPE: CStdGirder
START DISTANCE = 30-11-14
START SPAN CARRIED = 3-10-8
END DISTANCE = 34-0-0
END SPAN CARRIED = 3-10-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.13")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (1.13")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.28/1.00 (K-L:1) , BC=0.54/1.00 (N-O:1) , WB=0.91/1.00 (G-S:1) , SSI=0.12/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

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CONTINUED ON PAGE 2

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

PLATES (table is in Inches)

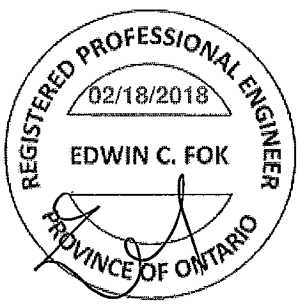
JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	2.00	2.25
B	TMWW-t	MT20	4.0	4.0	1.75	1.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TMWW-t	MT20	4.0	4.0	2.00	1.25
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMW+w	MT20	2.0	4.0		
G	TMWWW-t	MT20	5.0	6.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTW-m	MT20	4.0	4.0	1.75	2.00
J	TMWW-t	MT20	4.0	6.0	2.00	2.50
K	TMWW-t	MT20	4.0	6.0	1.75	1.50
L	TMVW-t	MT20	5.0	8.0	1.75	3.25
M	BMV1+t	MT20	6.0	9.0	Edge	0.50
N	BMWW+t	MT20	8.0	9.0	4.50	3.75
O	BMWW+t	MT20	4.0	6.0	3.00	1.75
P	BMWWW-t	MT20	5.0	6.0		
Q	BS-t	MT20	4.0	6.0		
R, U, V						
R	BMWW-t	MT20	4.0	4.0		
S	BBW-h	MT20	7.0	8.0		
T	BBWWW-m	MT20	5.0	8.0	2.75	3.50
W	BBWW-l	MT20	6.0	7.0		
X	BBWW-l	MT20	4.0	6.0	2.00	4.50
Y	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 6001.3 lbs FACTORED DOWN AT 30-11-14 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI	MAX (LC)	WEBS MAX. MEMB. FR-TO	FACTORED FORCE (LBS)	MAX UNBRAC LENGTH	MAX CSI (LC)
O- N	0 / 5941	-18.5	-18.5	0.54	(1)			10.00	
N- M	0 / 0	-63.7	-63.7	0.16	(1)			10.00	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
N	30-11-14	-6001	-6001	---	FRONT	VERT	TOTAL

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (I) (INPUT = 0.90)
JSI METAL= 0.73 (L) (INPUT = 1.00)

A:180236584(2)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292614	H3T	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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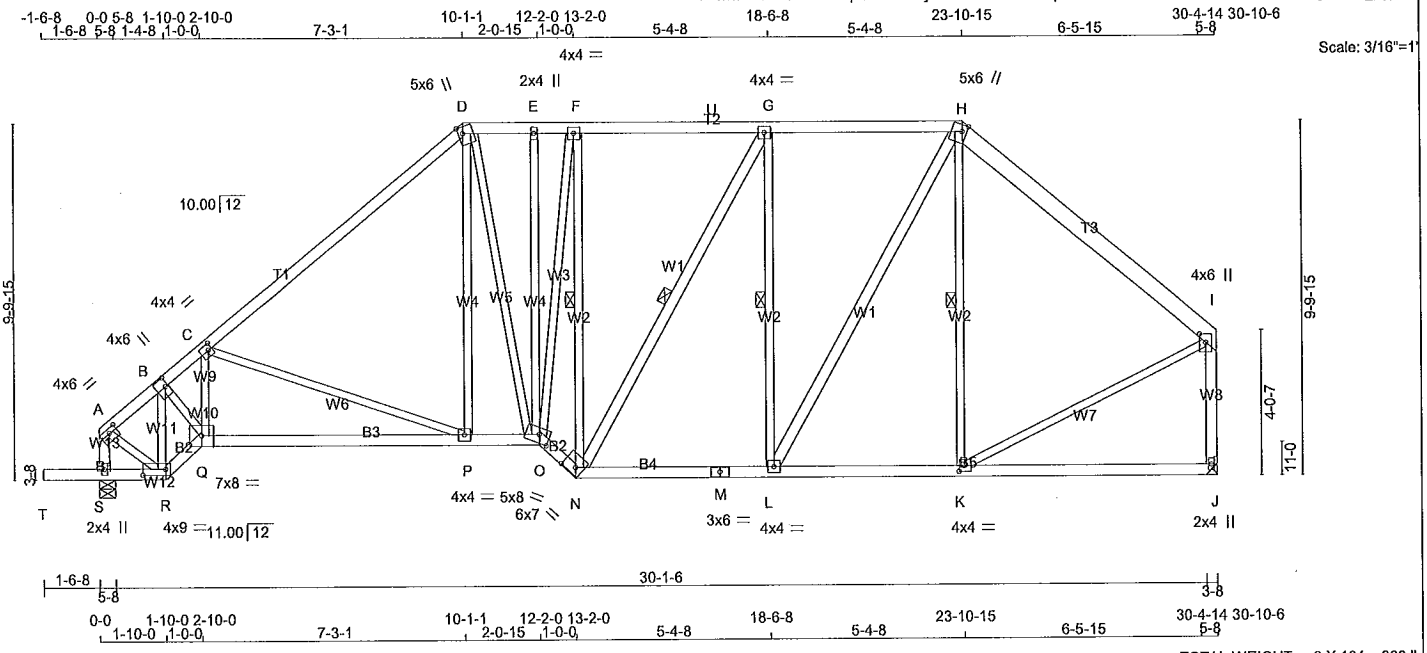


LUMBER

N. L. G. A. RULES

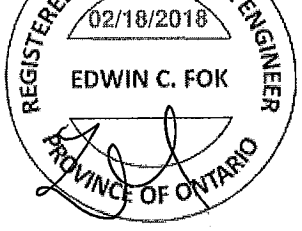
CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - I	2x6	DRY	No.2	SPF
S - A	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
R - Q	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
N - G	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0	1.50	2.75
B	TMWW+t	MT20	4.0	4.0	3.00	1.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW+m	MT20	5.0	6.0	2.25	1.50
I	TMVW+p	MT20	4.0	6.0	2.75	2.00
J	BMV1+p	MT20	2.0	4.0		
K	BMWW-t	MT20	4.0	4.0	2.00	1.75
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BBWW-h	MT20	6.0	7.0	2.25	4.50
O	BBWWW-m	MT20	5.0	8.0	2.75	3.50
P	BMWW-t	MT20	4.0	4.0		
Q	BBWW-l	MT20	7.0	8.0		
R	BBWW-l	MT20	4.0	9.0	2.00	7.50
S	BMV1+p	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
S	1688	0	1688	0	0	5-8	2-8
J	1542	0	1542	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-11	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1207	714 / 0	0 / 0	0 / 0	0 / 0	493 / 0	0 / 0
J	1104	646 / 0	0 / 0	0 / 0	0 / 0	458 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S

BRACING

FOR SECTION D-H, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.30 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-N, G-N, G-L, H-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1270 / 0	-78.0	-78.0	0.13 (1)	5.58
B- C	-1931 / 0	-78.0	-78.0	0.47 (1)	4.30
C- D	-1640 / 0	-78.0	-78.0	0.62 (1)	4.52
D- E	-1327 / 0	-85.5	-85.5	0.11 (1)	2.00
E- F	-1328 / 0	-85.5	-85.5	0.17 (1)	2.00
F- U	-1253 / 0	-85.5	-85.5	0.30 (1)	2.00
U- G	-1253 / 0	-85.5	-85.5	0.30 (1)	2.00
G- H	-1244 / 0	-85.5	-85.5	0.30 (1)	2.00
H- I	-1178 / 0	-78.0	-78.0	0.25 (1)	6.25
S- A	-1460 / 0	0.0	0.0	0.15 (1)	6.77
J- I	-1489 / 0	0.0	0.0	0.36 (1)	6.71
T- S	0 / 0	-96.5	-96.5	0.16 (1)	10.00
S- R	0 / 0	-18.5	-18.5	0.12 (1)	10.00
R- Q	0 / 1210	-18.5	-18.5	0.20 (1)	10.00
Q- P	0 / 1696	-18.5	-18.5	0.40 (1)	10.00
P- O	0 / 1233	-18.5	-18.5	0.36 (4)	10.00
O- N	0 / 1683	-18.5	-18.5	0.27 (1)	10.00
N- M	0 / 1245	-18.5	-18.5	0.26 (1)	10.00
M- L	0 / 1245	-18.5	-18.5	0.26 (1)	10.00
L- K	0 / 896	-18.5	-18.5	0.29 (4)	10.00
K- J	0 / 0	-18.5	-18.5	0.20 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R- B	-1405 / 0	0.24 (1)
B- Q	0 / 1389	0.31 (1)
Q- C	-251 / 0	0.04 (1)
C- P	-496 / 0	0.61 (1)
P- D	0 / 295	0.07 (4)
D- O	0 / 398	0.09 (1)
O- E	0 / 69	0.02 (1)
O- F	0 / 683	0.15 (1)
N- F	-1093 / 0	0.71 (1)
N- G	0 / 16	0.00 (1)
L- G	-524 / 0	0.34 (1)
L- H	0 / 706	0.11 (1)
K- H	-323 / 0	0.21 (1)
A- R	0 / 1098	0.25 (1)
K- I	0 / 1010	0.23 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.62/1.00 (C-D:1) , BC=0.40/1.00 (P-Q:1) , WB=0.71/1.00 (F-N:1) , SSI=0.28/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

A-18023655

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292614	J1T	3	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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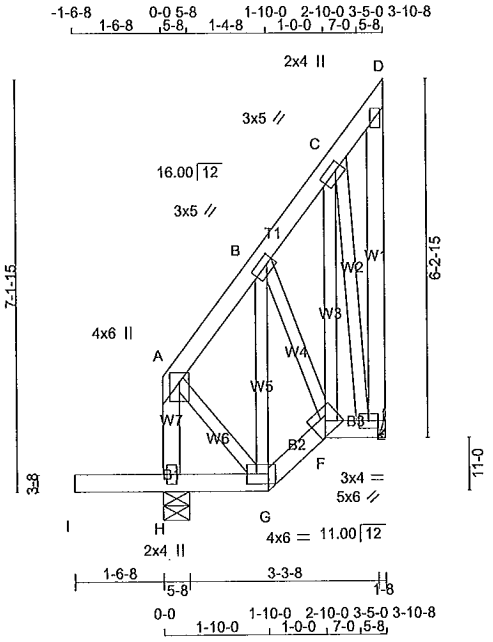


TOTAL WEIGHT = 3 X 37 = 111 lb

[M]

LUMBER

N. L. G. A. RULES

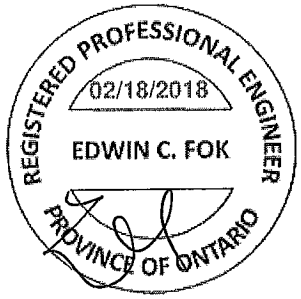
CHORDS	SIZE		LUMBER	DESCR.
H - A	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.00	2.00
B	TMWW-t	MT20	3.0	5.0	1.50	1.50
C	TMWW-t	MT20	3.0	5.0	1.50	1.50
D	TMV+p	MT20	2.0	4.0		
E	BMVW1-t	MT20	3.0	4.0		
F	BBWW-h	MT20	5.0	6.0		
G	BBWW-l	MT20	4.0	6.0	2.00	4.50
H	BMV1+p	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H	365	0	365	0	0	5-8	1-8
E	157	0	157	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	260	159 / 0	0 / 0	0 / 0	0 / 0	101 / 0	0 / 0
E	112	68 / -13	0 / 0	0 / 0	0 / 0	44 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- A	-137 / 0	0.0	0.0	0.02 (1)	7.81	A- G	-4 / 49	0.01 (1)
A- B	-52 / 6	-78.0	-78.0	0.03 (1)	6.25	G- B	-100 / 0	0.03 (1)
B- C	-49 / 0	-78.0	-78.0	0.03 (1)	6.25	B- F	-30 / 13	0.01 (1)
C- D	-4 / 0	-78.0	-78.0	0.01 (1)	10.00	F- C	-2 / 79	0.02 (1)
E- D	-24 / 0	0.0	0.0	0.02 (1)	7.81	C- E	-125 / 0	0.05 (1)
I- H	0 / 0	-96.5	-96.5	0.16 (1)	10.00			
H- G	0 / 0	-18.5	-18.5	0.16 (1)	10.00			
G- F	-5 / 47	-18.5	-18.5	0.01 (1)	10.00			
F- E	0 / 19	-18.5	-18.5	0.01 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 544 (0.03")

CSI: TC=0.03/1.00 (A-B:1) , BC=0.16/1.00 (G-H:1) , WB=0.05/1.00 (C-E:1) , SSI=0.12/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (E) (INPUT = 0.90)
JSI METAL= 0.04 (G) (INPUT = 1.00)

A-18023656

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292614	J1TA	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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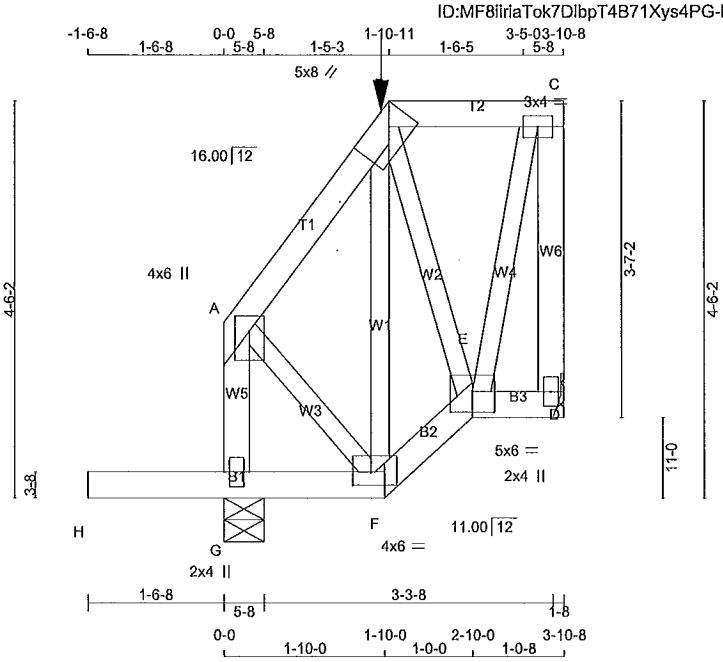


Scale = 1:26.2

TOTAL WEIGHT = 28 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
G - A	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in Inches)

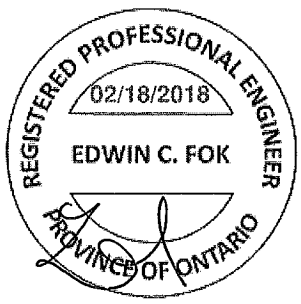
JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.00	2.00
B	TTWW-h	MT20	5.0	8.0	Edge	2.75
C	TMVW-t	MT20	3.0	4.0		
D	BMV1+p	MT20	2.0	4.0		
E	BBWW-I	MT20	5.0	6.0	2.75	3.00
F	BBWW-I	MT20	4.0	6.0	1.75	2.50
G	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 54.3 lbs FACTORED DOWN AT 1-10-11 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
D	248	0	248	0	0	IN-SX	IN-SX
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	
G	425	0	425	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	177	109 / -13	0 / 0	0 / 0	0 / 0	68 / 0	0 / 0
G	304	183 / 0	0 / 0	0 / 0	0 / 0	121 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-88 / 0	-78.0	-78.0	0.05 (1)	6.25	F- B	-120 / 0	0.03 (1)
B- C	-40 / 0	-111.3	-111.3	0.08 (1)	6.25	B- E	-51 / 8	0.01 (1)
D- C	-235 / 0	0.0	0.0	0.05 (1)	7.81	E- C	0 / 131	0.03 (1)
G- A	-190 / 0	0.0	0.0	0.02 (1)	7.81	A- F	0 / 69	0.02 (1)
H- G	0 / 0	-96.5	-96.5	0.17 (1)	10.00			
G- F	0 / 0	-26.4	-26.4	0.17 (1)	10.00			
F- E	0 / 73	-26.4	-26.4	0.02 (1)	10.00			
E- D	0 / 0	-26.4	-26.4	0.01 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
B	1-10-11	-54	-54	---	FRONT	VERT	TOTAL

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 1-10-11
RIGHT SETBACK = 0-0
END SETBACK = 3-8-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 555 (0.03")

CSI: TC=0.08/1.00 (B-C:1) , BC=0.17/1.00 (F-G:1) , WB=0.03/1.00 (B-F:1) , SSI=0.13/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

A-18023657

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292614	J1TB	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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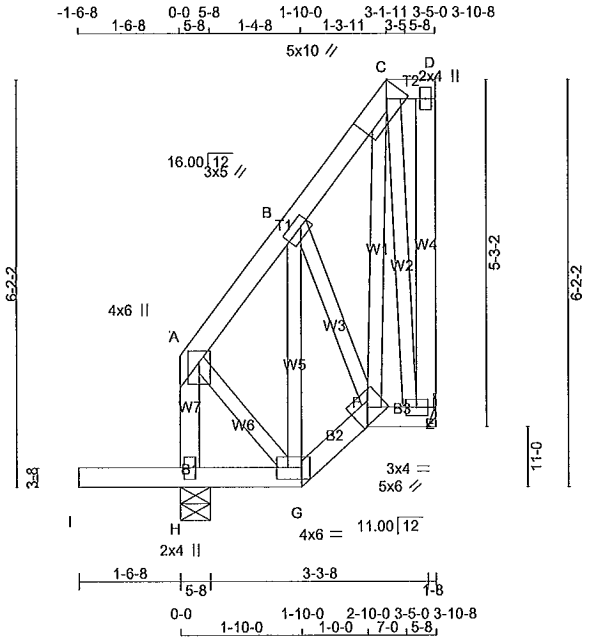


TOTAL WEIGHT = 36 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

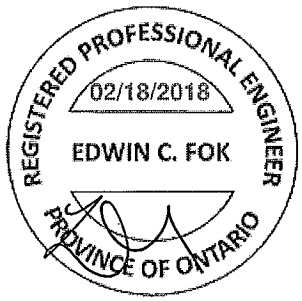
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.00	2.00
B	TMWW-t	MT20	3.0	5.0	1.50	1.50
C	TTWW-h	MT20	5.0	10.0	Edge	2.75
D	TMV+p	MT20	2.0	4.0		
E	BMVW1-t	MT20	3.0	4.0		
F	BBWW-h	MT20	5.0	6.0		
G	BBWW-t	MT20	4.0	6.0	2.00	4.50
H	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
E	157	0	157	0	0	IN-SX	IN-SX
						HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
H	365	0	365	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	112	68 / -13	0 / 0	0 / 0	0 / 0	44 / 0	0 / 0
H	260	159 / 0	0 / 0	0 / 0	0 / 0	101 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-52 / 6	-78.0	-78.0	0.03 (1)	6.25	G- B	-100 / 0	0.03 (1)
B- C	-51 / 0	-78.0	-78.0	0.03 (1)	6.25	B- F	-33 / 13	0.01 (1)
C- D	0 / 0	-78.0	-78.0	0.01 (1)	10.00	F- C	-2 / 82	0.02 (1)
E- D	-29 / 0	0.0	0.0	0.01 (1)	7.81	C- E	-121 / 0	0.05 (1)
H- A	-137 / 0	0.0	0.0	0.02 (1)	7.81	A- G	-4 / 49	0.01 (1)
I- H	0 / 0	-96.5	-96.5	0.16 (1)	10.00			
H- G	0 / 0	-18.5	-18.5	0.16 (1)	10.00			
G- F	-5 / 47	-18.5	-18.5	0.01 (1)	10.00			
F- E	0 / 18	-18.5	-18.5	0.01 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 544 (0.03")

CSI: TC=0.03/1.00 (A-B:1) , BC=0.16/1.00 (G-H:1) , WB=0.05/1.00 (C-E:1) , SSI=0.12/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.12 (E) (INPUT = 0.90)
JSI METAL= 0.04 (G) (INPUT = 1.00)

A-18023658

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292615	H1TB	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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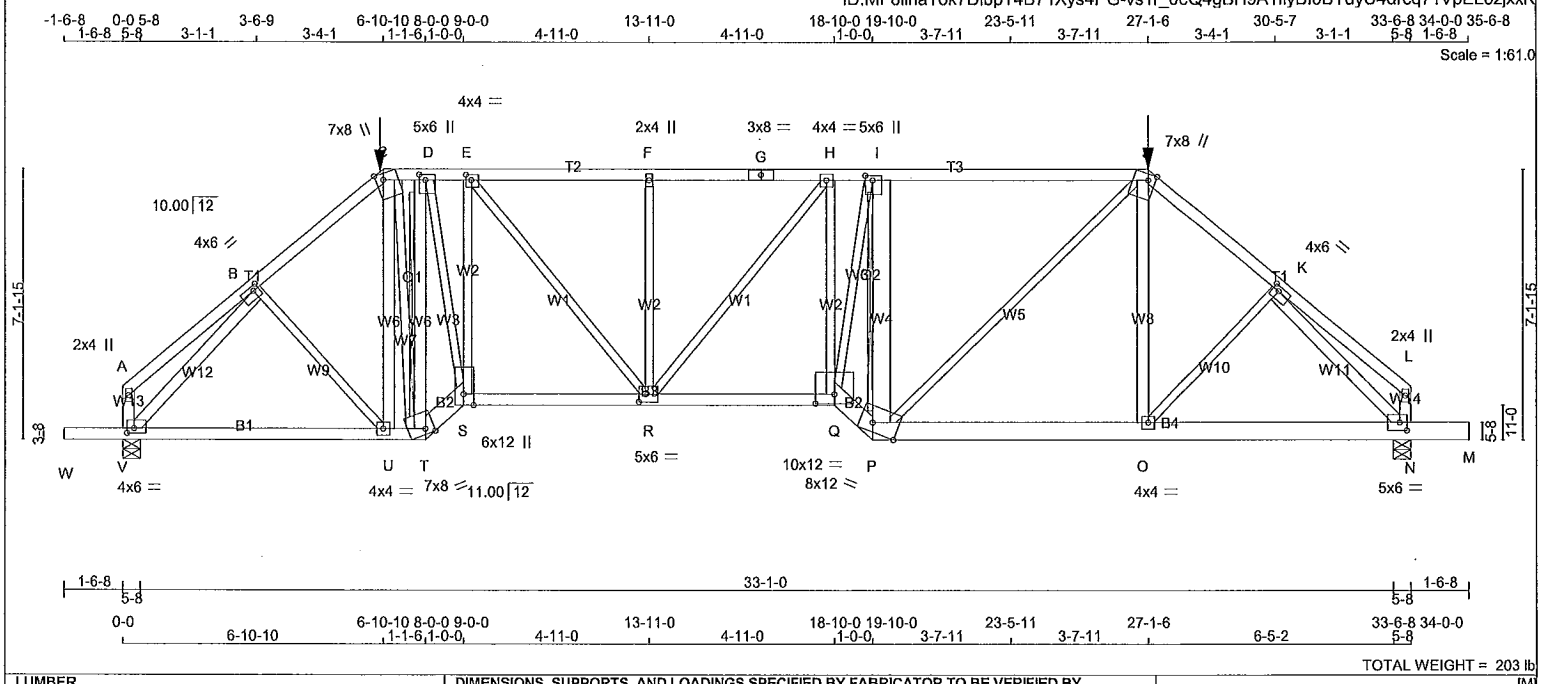

TOTAL WEIGHT = 203 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x4	DRY	2100F 1.8E	SPF
G - J	2x4	DRY	2100F 1.8E	SPF
J - L	2x4	DRY	No.2	SPF
V - A	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
W - T	2x4	DRY	No.2	SPF
T - S	2x6	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
U - C	2x4	DRY	No.2	SPF
T - D	2x4	DRY	No.2	SPF
P - I	2x6	DRY	No.2	SPF
O - J	2x4	DRY	No.2	SPF
V - B	2x4	DRY	No.2	SPF
K - N	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0		
B	TMWW-t	MT20	4.0	6.0	1.50	1.75
C	TTWW+m	MT20	7.0	8.0	Edge	2.25
D	TMWW+t	MT20	5.0	6.0	1.75	2.00
E	TMWW-t	MT20	4.0	4.0	1.75	1.75
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	8.0		
H	TMWW-t	MT20	4.0	4.0		
I	TMWW+t	MT20	5.0	6.0	1.50	2.25
J	TTWW+m	MT20	7.0	8.0	Edge	2.25
K	TMWW-t	MT20	4.0	6.0	1.50	2.00
L	TMV+p	MT20	2.0	4.0		
N	BMVW1-t	MT20	5.0	6.0	2.50	2.25
O	BMWW-t	MT20	4.0	4.0		
P	BBWW-m	MT20	8.0	12.0	Edge	8.00
Q	BBWW-l	MT20	10.0	12.0	3.00	6.00

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

REGISTERED PROFESSIONAL ENGINEER
02/18/2018
EDWIN C. FOK
PROVINCE OF ONTARIO

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	2523	0	2523	0	0	5-8	2-11
N	2521	0	2521	0	0	5-8	2-11

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	1799	1094 / 0	0 / 0	0 / 0	0 / 0	705 / 0	0 / 0
N	1798	1093 / 0	0 / 0	0 / 0	0 / 0	705 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.53 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT D-T, I-P

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 17	-78.0	-78.0	0.14 (1)		10.00
B- C	-2692 / 0	-78.0	-78.0	0.26 (1)		3.98
C- D	-2264 / 0	-114.5	-114.5	0.04 (1)		5.41
D- E	-2813 / 0	-114.5	-114.5	0.10 (1)		4.91
E- F	-3502 / 0	-78.0	-78.0	0.32 (1)		4.27
F- G	-3502 / 0	-78.0	-78.0	0.32 (1)		4.27
G- H	-3502 / 0	-78.0	-78.0	0.32 (1)		4.27
H- I	-3556 / 0	-114.5	-114.5	0.66 (1)		3.53
I- J	-3152 / 0	-114.5	-114.5	0.83 (1)		3.76
J- K	-2719 / 0	-78.0	-78.0	0.27 (1)		3.96
K- L	0 / 17	-78.0	-78.0	0.14 (1)		10.00
V- A	-107 / 0	0.0	0.0	0.01 (1)		7.81
N- L	-106 / 0	0.0	0.0	0.01 (1)		7.81
W- V	0 / 0	-96.5	-96.5	0.17 (1)		10.00
V- U	0 / 1906	-27.2	-27.2	0.58 (1)		10.00
U- T	0 / 2041	-27.2	-27.2	0.61 (1)		10.00
T- S	0 / 3127	-27.2	-27.2	0.41 (1)		10.00
S- R	0 / 2830	-63.7	-63.7	0.76 (1)		10.00
R- Q	0 / 3549	-63.7	-63.7	0.89 (1)		10.00
Q- P	0 / 4029	-27.2	-27.2	0.53 (1)		10.00
P- O	0 / 2071	-27.2	-27.2	0.38 (1)		10.00
O- N	0 / 1945	-27.2	-27.2	0.36 (1)		10.00
N- M	0 / 0	-96.5	-96.5	0.08 (1)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- U	0 / 210	0.05 (1)
U- C	-26 / 161	0.04 (4)
C- T	0 / 1329	0.33 (1)
T- D	-3394 / 0	0.99 (1)
D- S	0 / 3290	0.81 (1)
S- E	-956 / 0	0.61 (1)
E- R	0 / 1069	0.26 (1)
R- F	-495 / 0	0.31 (1)
R- H	-76 / 0	0.09 (1)
Q- H	0 / 346	0.09 (1)
Q- I	0 / 2555	0.63 (1)
P- I	-3552 / 0	0.74 (1)
O- J	0 / 182	0.05 (4)
O- K	0 / 182	0.05 (1)
V- B	-2907 / 0	0.94 (1)
K- N	-2930 / 0	0.90 (1)
P- J	0 / 1472	0.36 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	6-10-10	-213	-213	---	FRONT	VERT	TOTAL
J	27-1-6	-213	-213	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	21.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 6-10-10
END SETBACK = 3-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 9-0-0 OF SPAN MEASURED FROM THE LEFT.

GIRDER TYPE: CStdGirder
START DISTANCE = 9-0-0
START SPAN CARRIED = 3-10-8
END DISTANCE = 18-10-0
END SPAN CARRIED = 3-10-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 6-10-10
END SETBACK = 3-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 15-2-0 OF SPAN MEASURED FROM THE RIGHT.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.13")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.23")
ALLOWABLE DEFL.(TL)= L/360 (1.13")
CALCULATED VERT. DEFL.(TL) = L/ 904 (0.45")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

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CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292615	H1TB	1	1	TRUSS DESC.	

PLATES (table Is In Inches)

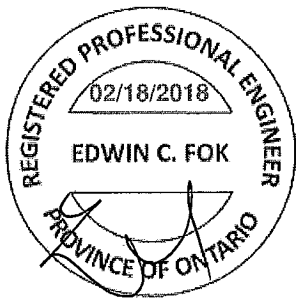
JT	TYPE	PLATES	W	LEN	Y	X
R	BMWWW-t	MT20	5.0	6.0	2.50	2.00
S	BBWW+p	MT20	6.0	12.0	Edge	3.25
T	BBWW-m	MT20	7.0	8.0	Edge	2.75
U	BMWW-t	MT20	4.0	4.0		
V	BMVW1-t	MT20	4.0	6.0	1.50	2.25

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 212.5 lbs FACTORED DOWN AT 6-10-10, AND 212.5 lbs FACTORED DOWN AT 27-1-6 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



CSI: TC=0.83/1.00 (I-J:1) , BC=0.89/1.00 (Q-R:1) , WB=0.99/1.00 (D-T:1) , SSI=0.49/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (K) (INPUT = 0.90)
JSI METAL= 0.73 (K) (INPUT = 1.00)

A-18023659(2)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292615	H2TB	5	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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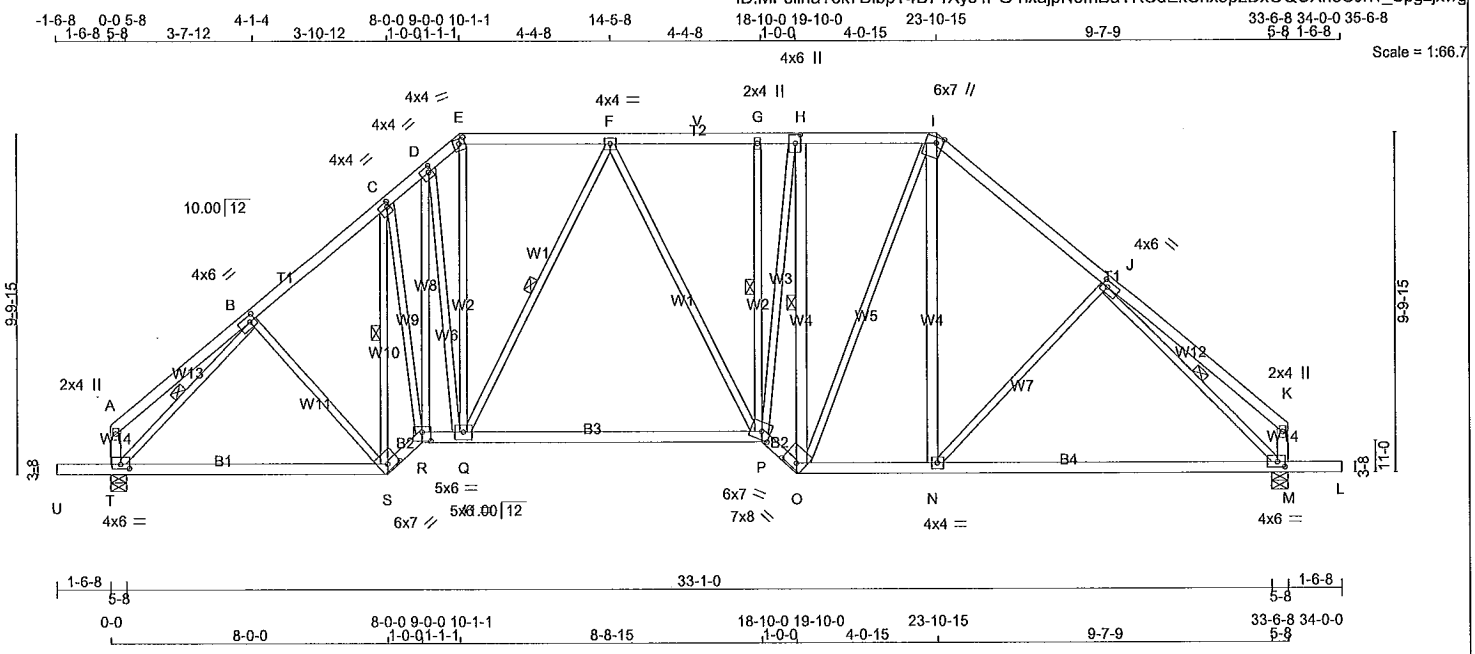

TOTAL WEIGHT = 5 X 201 = 1007 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
T - A	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
U - S	2x4	DRY	No.2	SPF
S - R	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
O - H	2x4	DRY	No.2	SPF
N - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0		
B	TMWW-t	MT20	4.0	6.0	2.00	2.00
C	TMWW-t	MT20	4.0	4.0	1.75	1.00
D	TMWW-t	MT20	4.0	4.0	2.00	1.25
E	TTW-m	MT20	4.0	4.0	1.75	2.00
F	TMWW-t	MT20	4.0	4.0		
G	TMW+w	MT20	2.0	4.0		
H	TMWW+t	MT20	4.0	6.0	3.00	1.75
I	TTWW+m	MT20	6.0	7.0	2.00	2.25
J	TMWW-t	MT20	4.0	6.0	2.00	2.00
K	TMV+p	MT20	2.0	4.0		
M	BMVW1-t	MT20	4.0	6.0	1.75	2.50
N	BMWW-t	MT20	4.0	4.0		
O	BBWW-h	MT20	7.0	8.0	2.00	5.00
P	BBWWW-m	MT20	6.0	7.0	2.75	3.00
Q	BMWWW-t	MT20	5.0	6.0		
R	BBWW-l	MT20	5.0	6.0	3.00	3.00
S	BBWW-h	MT20	6.0	7.0	2.00	4.00
T	BMVW1-t	MT20	4.0	6.0	1.50	3.00

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

REGISTERED PROFESSIONAL ENGINEER
02/18/2018
EDWIN C. FOK
PROVINCE OF ONTARIO

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	1841	0	1841	0	0	5-8	1-15
M	1841	0	1841	0	0	5-8	1-15

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1317	778 / 0	0 / 0	0 / 0	0 / 0	538 / 0	0 / 0
M	1317	778 / 0	0 / 0	0 / 0	0 / 0	538 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, M

BRACING

FOR SECTION E-I, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.57 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-S, F-Q, G-P, H-O, B-T, J-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 20	-78.0	-78.0	0.17 (1)	10.00
B- C	-1779 / 0	-78.0	-78.0	0.18 (1)	4.87
C- D	-2033 / 0	-78.0	-78.0	0.20 (1)	4.57
D- E	-1762 / 0	-78.0	-78.0	0.19 (1)	4.85
E- F	-1439 / 0	-85.5	-85.5	0.22 (1)	2.00
F- V	-1685 / 0	-85.5	-85.5	0.23 (1)	2.00
V- G	-1685 / 0	-85.5	-85.5	0.23 (1)	2.00
G- H	-1684 / 0	-85.5	-85.5	0.18 (1)	2.00
H- I	-1514 / 0	-85.5	-85.5	0.18 (1)	2.00
I- J	-1717 / 0	-78.0	-78.0	0.30 (1)	4.80
J- K	0 / 29	-78.0	-78.0	0.32 (1)	10.00
T- A	-123 / 0	0.0	0.0	0.01 (1)	7.81
M- K	-155 / 0	0.0	0.0	0.02 (1)	7.81
U- T	0 / 0	-96.5	-96.5	0.16 (1)	10.00
T- S	0 / 1354	-18.5	-18.5	0.48 (4)	10.00
S- R	0 / 1862	-18.5	-18.5	0.30 (1)	10.00
R- Q	0 / 1447	-18.5	-18.5	0.51 (4)	10.00
Q- P	0 / 1673	-18.5	-18.5	0.53 (4)	10.00
P- O	0 / 2016	-18.5	-18.5	0.33 (1)	10.00
O- N	0 / 1297	-18.5	-18.5	0.56 (4)	10.00
N- M	0 / 1396	-18.5	-18.5	0.56 (4)	10.00
M- L	0 / 0	-96.5	-96.5	0.16 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- S	0 / 55	0.02 (4)
S- C	-1179 / 0	0.48 (1)
C- R	0 / 707	0.16 (1)
R- D	0 / 475	0.11 (1)
D- Q	-180 / 16	0.24 (1)
Q- E	0 / 858	0.19 (1)
Q- F	-526 / 0	0.35 (1)
F- P	0 / 33	0.01 (4)
P- G	-76 / 0	0.04 (1)
P- H	0 / 1503	0.34 (1)
O- H	-1857 / 0	0.86 (1)
O- I	0 / 548	0.12 (1)
N- I	0 / 293	0.05 (4)
N- J	-150 / 9	0.16 (1)
T- B	-2020 / 0	0.51 (1)
J- M	-2020 / 0	0.74 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	21.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.13")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (1.13")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.35")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.32/1.00 (J-K:1) , BC=0.56/1.00 (N-O:4) , WB=0.86/1.00 (H-O:1) , SSI=0.19/1.00 (Q-R:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.50 (B) (INPUT = 1.00)

A-18023660

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292615	H2TC	1	2	TRUSS DESC.	

Alpa Roof Truss, Maple

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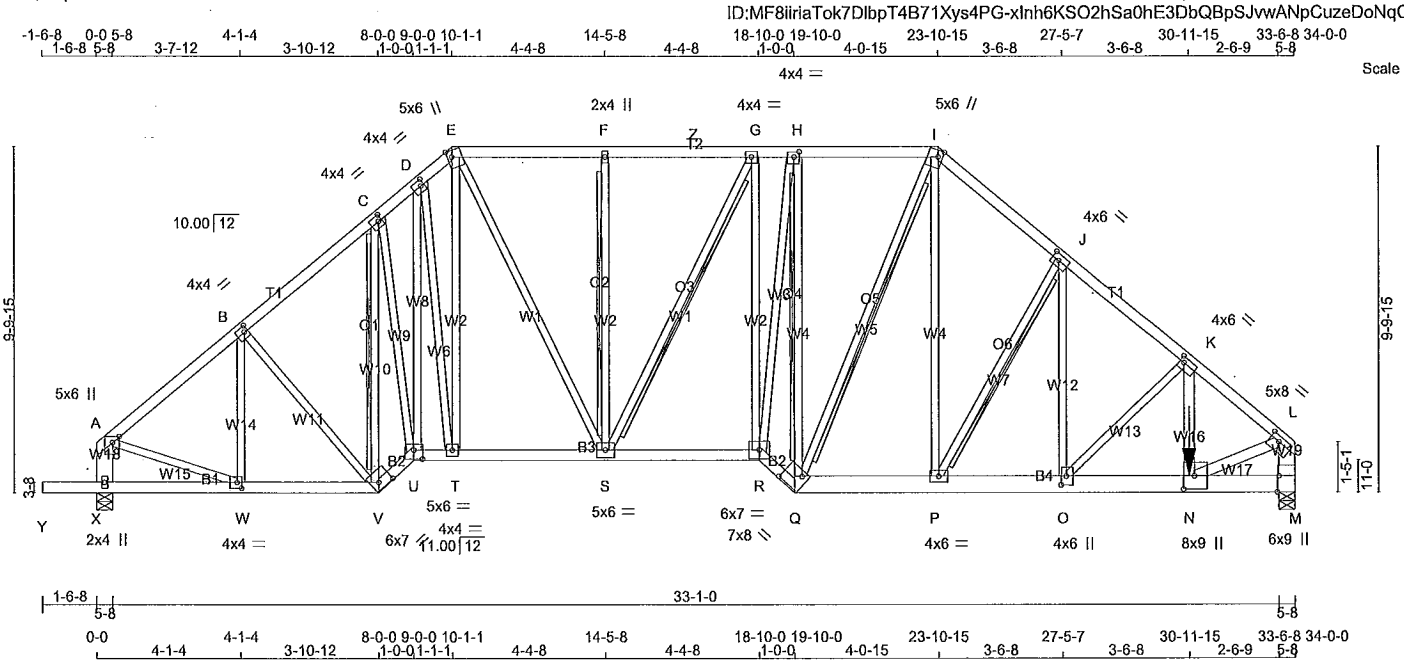


TOTAL WEIGHT = 2 X 222 = 445 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
X - A	2x6	DRY	No.2	SPF
M - L	2x6	DRY	No.2	SPF
Y - V	2x4	DRY	No.2	SPF
V - U	2x4	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
R - Q	2x4	DRY	No.2	SPF
Q - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF

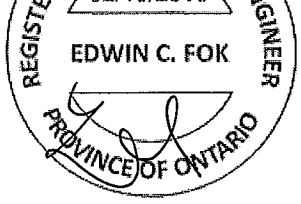
DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- E	1	12	TOP
E- I	1	12	TOP
I- L	1	12	TOP
X- A	2	12	TOP
M- L	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
Y- V	1	12	TOP
V- U	1	12	TOP
U- R	1	12	TOP
R- Q	1	12	TOP
Q- M	2	12	SIDE(165.4)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
K- N	2	2	SIDE(1405.0)
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	7293	0	7293	0	0	5-8	5-8
X	2377	0	2377	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	5104	3651 / 0	0 / 0	0 / 0	0 / 0	1453 / 0	0 / 0
X	1690	1060 / 0	0 / 0	0 / 0	0 / 0	629 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, X

BEARING SIZE FACTOR = 1.15 AT JNT(S) M (BASED ON SUPPORT DEPTH = 1-8)

BRACING

FOR SECTION E-I, MAX. PURLIN SPACING = 2.00 FT.

FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.36 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT C-V, F-S, G-S, I-Q, J-P

2x6 DRY SPF No.2 T-BRACE AT H-Q

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX. (PLF) CSI (LC)	MAX. UNBRAC LENGTH
A- B	-2430 / 0	-78.0	-78.0	0.13 (1)	5.65
B- C	-2504 / 0	-78.0	-78.0	0.13 (1)	5.59
C- D	-2840 / 0	-78.0	-78.0	0.12 (1)	5.32
D- E	-2593 / 0	-78.0	-78.0	0.12 (1)	5.52
E- F	-2583 / 0	-85.5	-85.5	0.17 (1)	2.00
F- Z	-2583 / 0	-85.5	-85.5	0.17 (1)	2.00
Z- G	-2583 / 0	-85.5	-85.5	0.17 (1)	2.00
G- H	-2860 / 0	-85.5	-85.5	0.13 (1)	2.00
H- I	-2642 / 0	-85.5	-85.5	0.13 (1)	2.00
I- J	-3453 / 0	-78.0	-78.0	0.12 (1)	4.94
J- K	-4782 / 0	-78.0	-78.0	0.17 (1)	4.29
K- L	-7736 / 0	-78.0	-78.0	0.28 (1)	3.36
X- A	-2195 / 0	0.0	0.0	0.08 (1)	7.81
M- L	-7053 / 0	0.0	0.0	0.25 (1)	5.63
Y- X	0 / 0	-96.5	-96.5	0.09 (1)	10.00
X- W	0 / 0	-18.5	-18.5	0.04 (4)	10.00
W- V	0 / 1883	-18.5	-18.5	0.19 (1)	10.00
V- U	0 / 2621	-18.5	-18.5	0.23 (1)	10.00
U- T	0 / 2080	-18.5	-18.5	0.23 (1)	10.00
T- S	0 / 2051	-18.5	-18.5	0.23 (1)	10.00
S- R	0 / 2854	-18.5	-18.5	0.28 (1)	10.00
R- Q	0 / 3478	-18.5	-18.5	0.31 (1)	10.00
Q- P	0 / 2653	-18.5	-18.5	0.18 (1)	10.00
P- O	0 / 3688	-18.5	-18.5	0.25 (1)	10.00
O- N	0 / 5943	-18.5	-18.5	0.54 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
W- B	-428 / 0	0.08 (1)
B- V	0 / 49	0.01 (1)
V- C	-1755 / 0	0.52 (1)
C- U	0 / 1260	0.16 (1)
U- D	0 / 539	0.07 (1)
D- T	-300 / 0	0.20 (1)
T- E	0 / 361	0.04 (1)
E- S	0 / 1175	0.15 (1)
S- F	-440 / 0	0.17 (1)
S- G	-611 / 0	0.31 (1)
R- G	0 / 466	0.06 (1)
R- H	0 / 1902	0.24 (1)
Q- H	-2238 / 0	0.65 (1)
Q- I	-26 / 99	0.02 (4)
P- I	0 / 1972	0.24 (1)
P- J	-2150 / 0	0.57 (1)
O- J	0 / 2405	0.30 (1)
O- K	-3175 / 0	0.65 (1)
N- K	0 / 3886	0.34 (1)
A- W	0 / 1948	0.24 (1)
N- L	0 / 6265	0.55 (1)

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

GIRDER TYPE: CStdGirder
START DISTANCE = 30-11-14
START SPAN CARRIED = 3-10-8
END DISTANCE = 34-0-0
END SPAN CARRIED = 3-10-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.13")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (1.13")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.28/1.00 (K-L:1) , BC=0.54/1.00 (N-O:1) , WB=0.65/1.00 (K-O:1) , SSI=0.10/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

A-18023661

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292615	H2TC	1	2	TRUSS DESC.	

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

PLATES (table is in Inches)

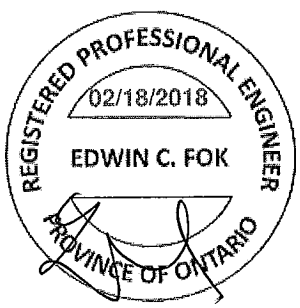
JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	2.00	2.25
B, C, D						
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMW+w	MT20	2.0	4.0		
G	TMWW-t	MT20	4.0	4.0		
H	TMWW-t	MT20	4.0	4.0	1.75	1.75
I	TTWW+m	MT20	5.0	6.0	2.25	1.50
J	TMWW-t	MT20	4.0	6.0	2.00	2.50
K	TMWW-t	MT20	4.0	6.0	1.75	1.50
L	TMVW-t	MT20	5.0	8.0	1.75	3.25
M	BMV1+t	MT20	6.0	9.0	Edge	0.50
N	BMWW+t	MT20	8.0	9.0	4.50	3.75
O	BMWW+t	MT20	4.0	6.0	3.00	1.75
P	BMWW-t	MT20	4.0	6.0		
Q	BBWW-h	MT20	7.0	8.0	5.25	5.75
R	BBWW-l	MT20	6.0	7.0		
S	BMWWW-t	MT20	5.0	6.0		
T	BMWW-t	MT20	4.0	4.0		
U	BBWW-l	MT20	5.0	6.0	3.00	3.00
V	BBWW-h	MT20	6.0	7.0	2.25	4.50
W	BMWW-t	MT20	4.0	4.0	2.00	1.50
X	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 6001.3 lbs FACTORED DOWN AT 30-11-14 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)		MAX CSI (LC)	WEBS MAX. MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO		UNBRAC LENGTH FR-TO		
N- M	0 / 0	-63.7	-63.7	0.16 (1)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
N	30-11-14	-6001	-6001	---	FRONT	VERT	TOTAL

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

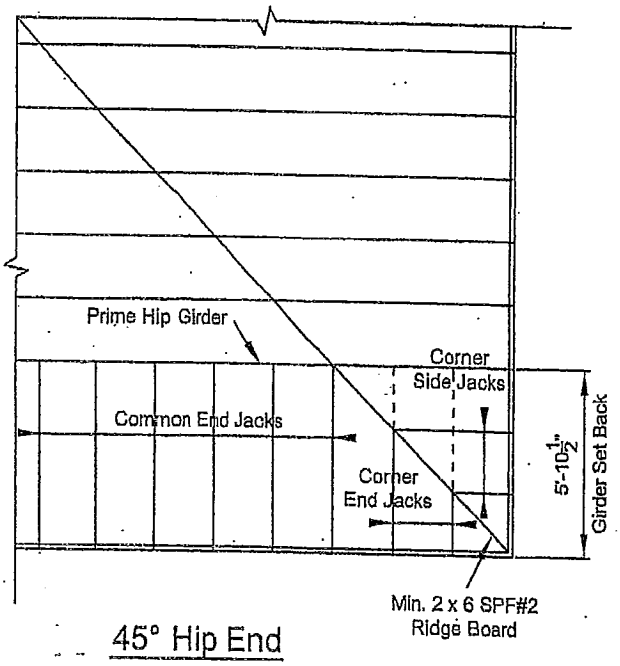
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (P) (INPUT = 0.90)
JSI METAL= 0.73 (L) (INPUT = 1.00)

A-1802366 1(2)

STRACON ENGINEERING INC.



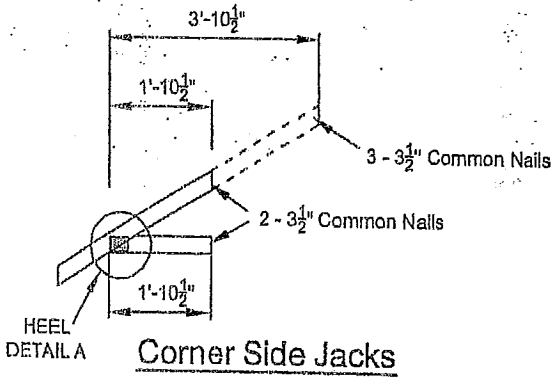
45° Hip End

LUMBER SPECIFICATION

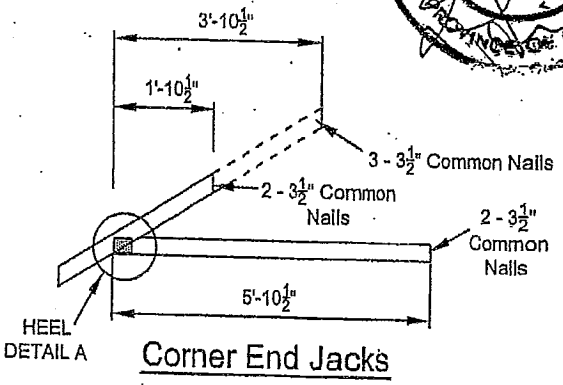
TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

DESIGN LOAD

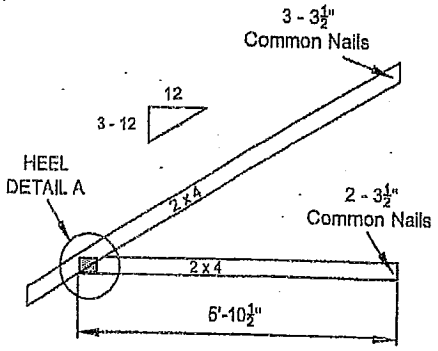
TOP CHORD SNOW LOAD	:	40.5 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD	:	50.5 P.S.F



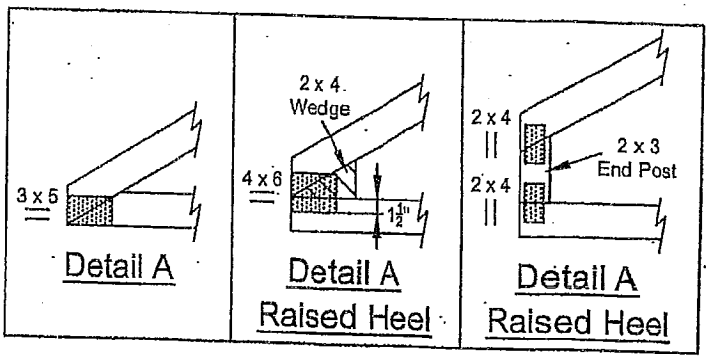
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

CS-51008

TECHNICAL BULLETIN

LUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

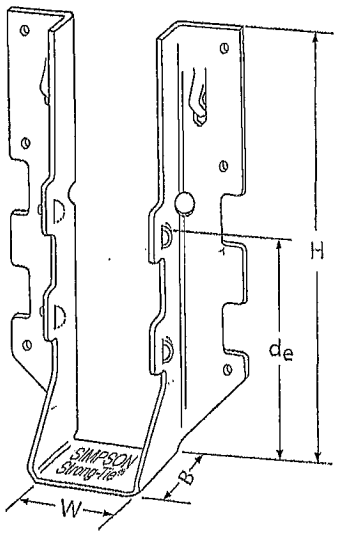
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

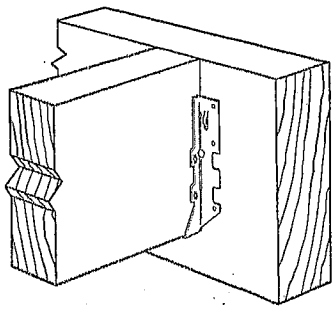
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified

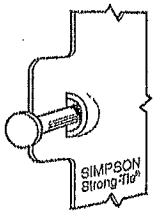


LUS28


Typical LUS Installation

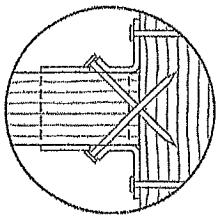
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LUS24	18	1 9/16	3 1/8	1 3/4	1 15/16	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 15/16	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1 9/16	4 3/4	1 3/4	3 5/8	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 7/8	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4 5/8	4 9/16	2	3 1/4	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1 9/16	6 5/8	1 3/4	3 3/4	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4 5/8	6 1/4	2	3 1/4	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1 9/16	7 13/16	1 3/4	3 7/8	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4 5/8	8 9/16	2	5 1/4	(8) 16d	(6) 16d	2580	3345	2320	2375

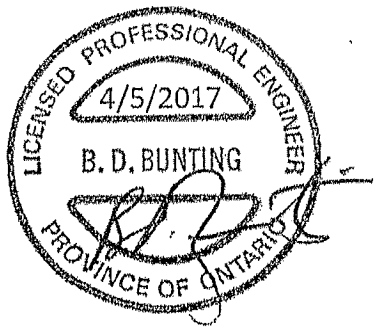
1. d_e is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580


Double Shear Nailing Top View.



LIMIT STATES DESIGN

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T-SPECLUS17 3/17 exp. 6/19

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

HHUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

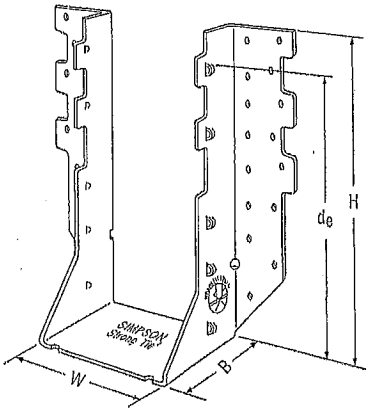
All HHUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 14 gauge

Finish: G90 galvanized

Design:

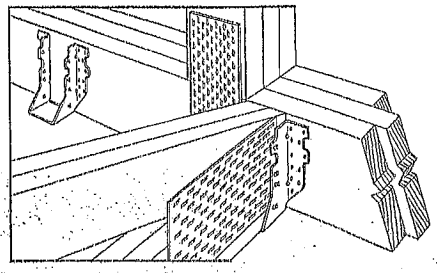
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.



HHUS410

Installation:

- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

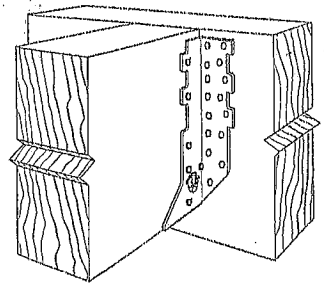

Typical HHUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)

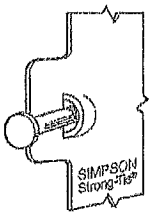
Options:

- See current catalogue for options

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
HHUS26-2	14	3 5/16	5 13/16	3	3 15/16	(14) 16d	(6) 16d	2850	7335	2065	5205
HHUS28-2	14	3 5/16	7 7/32	3	6 5/32	(22) 16d	(8) 16d	3765	8940	2675	6345
HHUS210-2	14	3 5/16	9 3/32	3	8	(30) 16d	(10) 16d	4745	9660	4310	7000
HHUS210-3	14	4 1/16	9	3	7 15/16	(30) 16d	(10) 16d	4745	10545	4310	7485
HHUS210-4	14	6 1/8	8 29/32	3	7 27/32	(30) 16d	(10) 16d	4745	10545	4310	7485
HHUS46	14	3 5/8	5 13/32	3	3 15/16	(14) 16d	(6) 16d	2540	7335	2065	5205
HHUS48	14	3 5/8	7 1/8	3	6 1/8	(22) 16d	(8) 16d	3765	8945	2267	6345
HHUS410	14	3 5/8	9	3	8	(30) 16d	(10) 16d	4745	9855	4310	7000
HHUS5.50/10	14	5 1/2	9	3	8	(30) 16d	(10) 16d	4745	10545	4310	7485
HHUS7.25/10	14	7 1/4	9	3 5/16	7 29/32	(30) 16d	(10) 16d	4745	10770	4310	7650

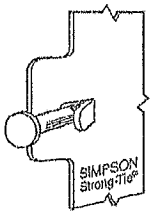
1. d_e is the distance from the seat of the hanger to the highest joist nail.


Typical HHUS Installation

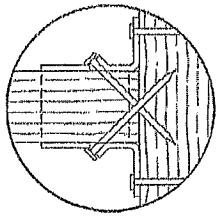


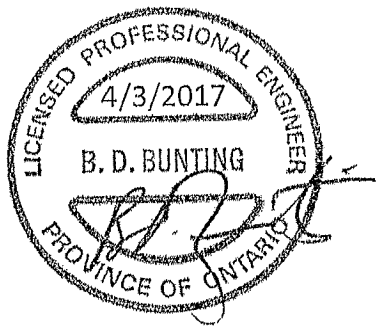
Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,680



Double Shear Nailing Side View. Do not bend tab back.


Double Shear Nailing Top View.



LIMIT STATES DESIGN

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T-SPECHHUS17 3/17 exp. 6/19

(800) 999-5099
strongtie.com

HUS/LJS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

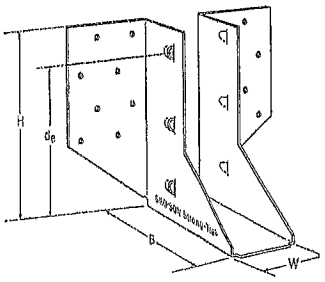
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

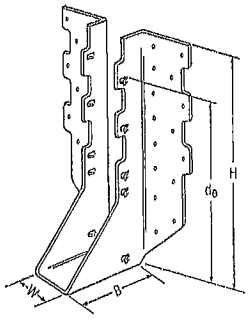
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

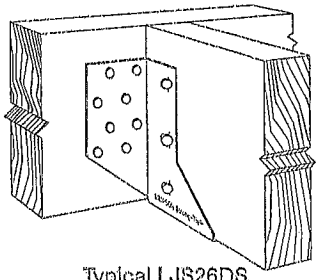
- See current catalogue for options



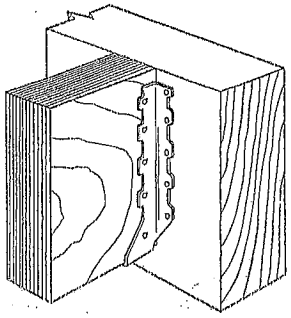
LJS26DS



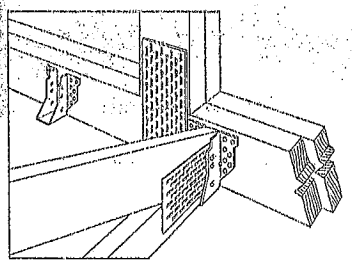
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS Installation



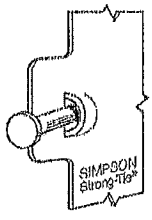
Typical HUS Installation



Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

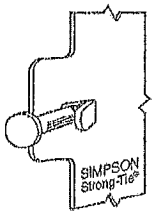
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
								lb.	lb.	lb.	lb.
LJS26DS	18	1 9/16	5	3½	4⅝	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1⅝	5⅝	3	3 15/16	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1⅝	7 3/32	3	6 9/32	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1⅝	9 3/32	3	7 31/32	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1 13/16	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

1. d_e is the distance from the seat of the hanger to the highest joist nail.

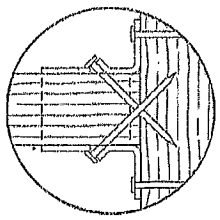


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

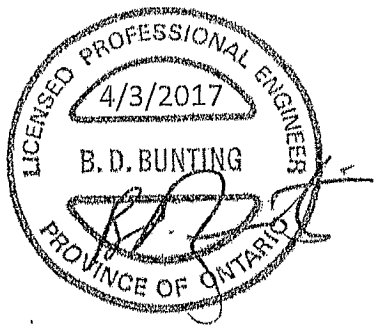
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



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(800) 999-5099
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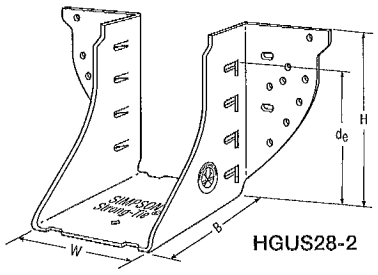
TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.



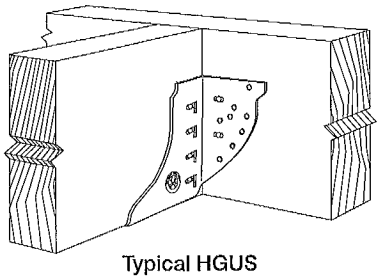
HGUS28-2

Material: 12 gauge

Finish: G90 galvanized

Design:

- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

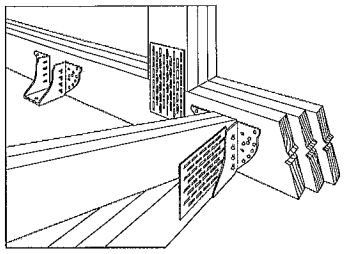

Typical HGUS Installation

Installation:

- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

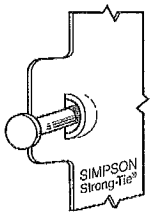
Options:

- See current catalogue for options


Typical HGUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)

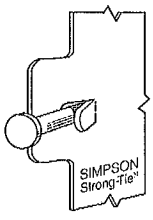
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
HGUS26	12	1⅝	5⅜	5	4 5/32	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3 5/16	5 7/16	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4 15/16	5½	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6 5/16	5 7/16	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1⅝	7⅛	5	6⅛	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3 5/16	7 3/16	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4 15/16	7¼	4	6⅜	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6 5/16	7 3/16	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210-2	12	3 5/16	9 3/16	4	8⅛	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-3	12	4 15/16	9¼	4	8⅜	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6 5/16	9 3/16	4	8⅛	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6 5/16	10⅝	4	10⅛	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6 5/16	12⅝	4	11⅛	(66) 16d	(22) 16d	10130	16400	7195	11645

1. d_e is the distance from the seat of the hanger to the highest joist nail.

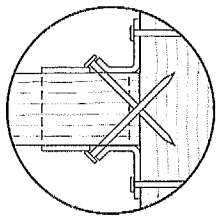


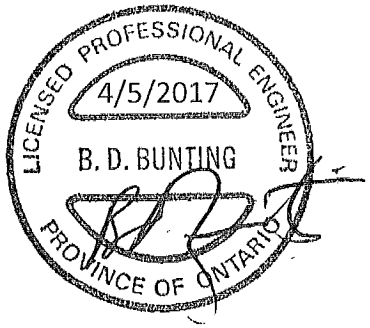
Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.


Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2019, and reflects information available as of March 1, 2017. This information is updated periodically and should not be relied upon after June 30, 2019. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

(800) 999-5099
strongtie.com