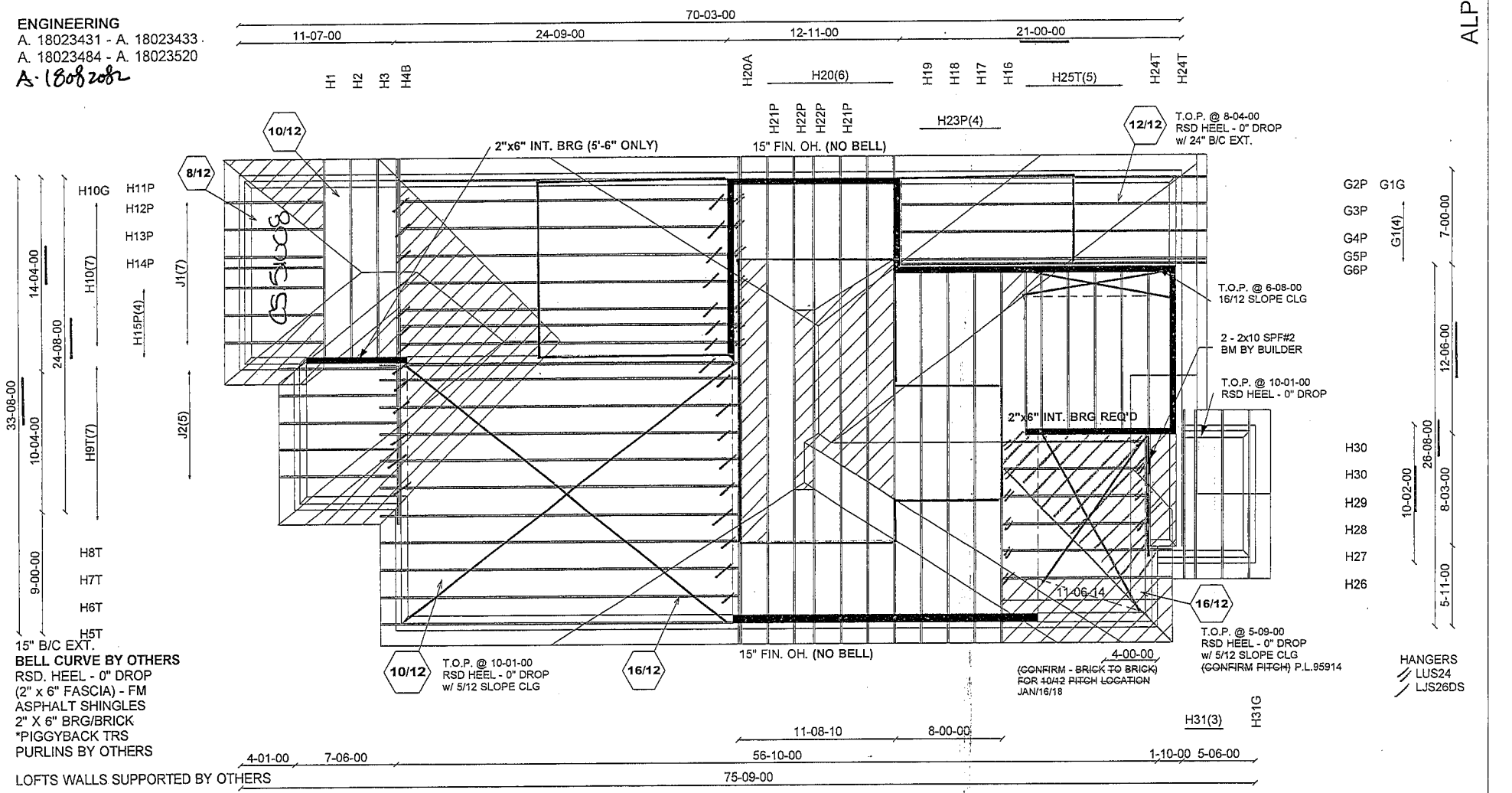
ALPA-BARRIE

ENGINEERING
A. 18023431 - A. 18023433
A. 18023484 - A. 18023520
A-18082082



15" B/C EXT.
BELL CURVE BY OTHERS
RSD. HEEL - 0" DROP
(2" x 6" FASCIA) - FM
ASPHALT SHINGLES
2" X 6" BRG/BRICK
*PIGGYBACK TRS
PURLINS BY OTHERS

LOFTS WALLS SUPPORTED BY OTHERS

~~CONFIRM TRUSS LAYOUT/DIRECTION~~
P.L. 95914

MAPLEWOOD

Mitek V. 8.2.0

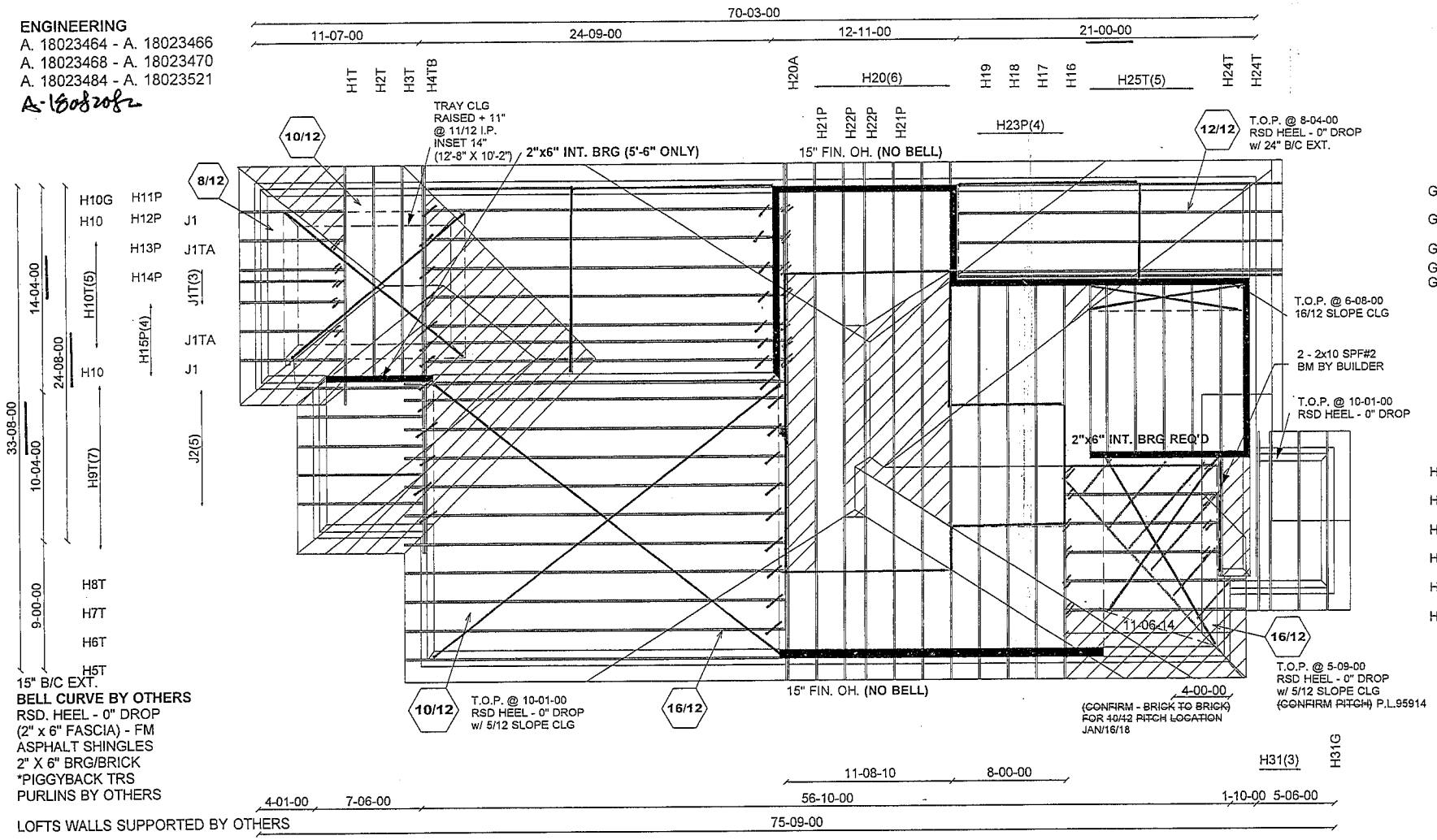
ALL CONVENTIONAL FRAMING TO CONFORM WITH PART 9 O.B.C. LATEST EDITION. ROOF RAFTERS THAT CROSS OVER TRUSSES TO BE 2X4 SPF #2 @ 24" O.C WITH A VERT. POST TO THE TRUSS UNDERNEATH AT EACH CROSS POINT. VERT. POSTS LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END POINTS & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

CONVENTIONAL FRAMING BY OTHERS

Job Track: 45147	Builder / Location: GOLD PARK HOMES / VAUGHAN	Model / Elevation: 4201 REV2 / B
Layout ID: 301036	Project: PINE VALLEY	
Plan Log: 99352	Date: 8/13/2018 Designer: AMANDA	

ALPA-BARRIE

ENGINEERING
A. 18023464 - A. 18023466
A. 18023468 - A. 18023470
A. 18023484 - A. 18023521
A-180820R2



15" B/C EXT.
BELL CURVE BY OTHERS
RSD. HEEL - 0" DROP
(2" x 6" FASCIA) - FM
ASPHALT SHINGLES
2" X 6" BRG/BRICK
*PIGGYBACK TRS
PURLINS BY OTHERS

LOFTS WALLS SUPPORTED BY OTHERS

~~CONFIRM TRUSS LAYOUT/DIRECTION~~
P.L. 95914

MAPLEWOOD

Mitek V. 8.2.0

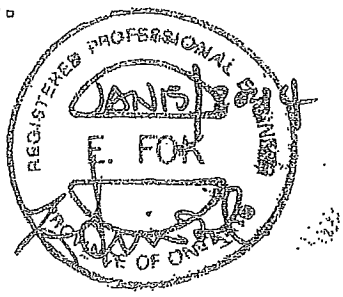
ALL CONVENTIONAL FRAMING TO CONFORM WITH PART 9 O.B.C. LATEST EDITION. ROOF RAFTERS THAT CROSS OVER TRUSSES TO BE 2X4 SPF #2 @ 24" O.C WITH A VERT. POST TO THE TRUSS UNDERNEATH AT EACH CROSS POINT. VERT. POSTS LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END POINTS & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

CONVENTIONAL FRAMING BY OTHERS

Job Track: 45147	Builder / Location: GOLD PARK HOMES / VAUGHAN	Model / Elevation: 4201 REV2 / B OPT TRAY
Layout ID: 301037	Project: PINE VALLEY	
Plan Log: 99352	Date: 8/13/2018 Designer: AMANDA	

Stracon Engineering Inc.

69 Graydon Crescent
Richmond Hill, Ontario
L4B 3W7
(905) 832-2250 Fax (905) 832-0286



RESPONSIBILITIES

1. Stracon Engineering Inc. is responsible for the design of trusses as individual components.
2. It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities with jurisdictions.
3. All dimensions are to be verified by owner contractor, architect or other authority before manufacture.
4. Stracon Engineering Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Stracon Engineering Inc. drawings is specified for the truss as a component only and forms an integral part of the truss design.
5. It is the truss manufacture's responsibility to ensure that trusses are manufactured in conformance with Stracon Engineering Inc. specifications outlined below.

SPECIFICATIONS

1. Trusses designed by Stracon Engineering Inc. conforms to the relevant section of the Ontario Building Code of Canada (Part 9 or Part 4) or to the Canadian code for farm buildings, whichever applies to the building type indicated on the drawings as well as to the procedures established by the truss plate institute of Canada. Unit stresses used are as per CSA-086-09.
2. Lumber is to be the sizes and grade specified.
3. Moisture content of lumber is not to exceed 19% in service unless otherwise specified.
4. Lumber not to be treated with chemicals unless otherwise specified.
5. Plates shall be applied to both faces of the truss at each joint and shall be positioned as specified.
6. The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals not exceeding 12.5 times in thickness.
7. Where not rigid ceiling is attached directly to the bottom chord, laterally brace the chords at intervals not exceeding 3M (10') o.c.

January 15, 2014

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
301032	H4B	1	2	TRUSS DESC.	

Alpa Roof Truss, Maple

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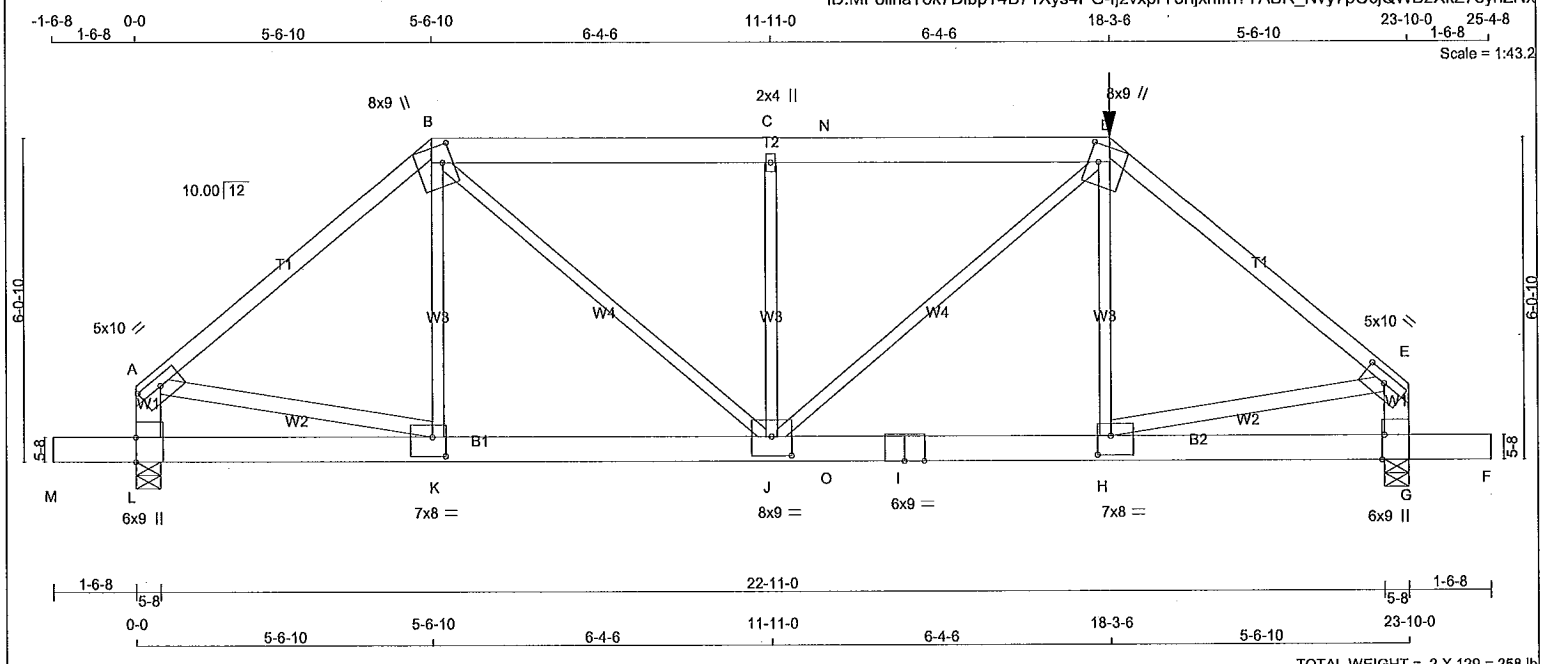


TOTAL WEIGHT = 2 X 129 = 258 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	1650F 1.5E	SPF
B - D	2x6	DRY	No.2	SPF
D - E	2x4	DRY	1650F 1.5E	SPF
L - A	2x6	DRY	No.2	SPF
G - E	2x6	DRY	No.2	SPF
M - I	2x6	DRY	1650F 1.5E	SPF
I - F	2x6	DRY	1650F 1.5E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
A - K	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

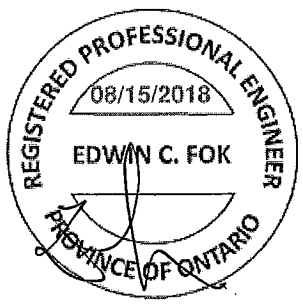
CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- B	1	12	TOP
D- E	1	12	SIDE(59.7)
B- D	2	12	SIDE(49.5)
L- A	2	12	TOP
G- E	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
M- I	2	8	SIDE(329.8)
I- F	2	8	SIDE(329.8)
WEBS : (0.122"X3") SPIRAL NAILS			
H- D	1	6	SIDE(94.0)
2x3	1	6	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

Note: Lateral brace(s) shown shall be 2x4 SPF#2.



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
L	8370	0	8370	0	0	5-8	5-8
G	8898	0	8898	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	5967	3645 / 0	0 / 0	0 / 0	0 / 0	2322 / 0	0 / 0
G	6348	3853 / 0	0 / 0	0 / 0	0 / 0	2495 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G
BEARING SIZE FACTOR = 1.15 AT JNT(S) L, G (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.50 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-8648 / 0	-78.0	-78.0	0.52 (1)	3.63		K- B	0 / 2666	0.33 (1)
B- C	-9493 / 0	-78.0	-78.0	0.26 (1)	3.81		B- J	0 / 3718	0.46 (1)
C- N	-9493 / 0	-78.0	-78.0	0.32 (1)	3.74		J- C	-700 / 0	0.18 (1)
N- D	-9493 / 0	-109.5	-109.5	0.32 (1)	3.74		J- D	0 / 3044	0.38 (1)
D- E	-9296 / 0	-78.0	-78.0	0.57 (1)	3.50		H- D	0 / 3015	0.37 (1)
L- A	-6918 / 0	0.0	0.0	0.25 (1)	5.68		A- K	0 / 6759	0.60 (1)
G- E	-7421 / 0	0.0	0.0	0.27 (1)	5.50		H- E	0 / 7266	0.64 (1)
M- L	0 / 0	-96.5	-96.5	0.03 (1)	10.00				
L- K	0 / 0	-583.4	-583.4	0.50 (1)	10.00				
K- J	0 / 6704	-583.4	-583.4	0.77 (1)	10.00				
J- O	0 / 7210	-583.4	-583.4	0.86 (1)	10.00				
O- I	0 / 7210	-678.2	-678.2	0.86 (1)	10.00				
I- H	0 / 7210	-678.2	-678.2	0.86 (1)	10.00				
H- G	0 / 0	-607.6	-607.6	0.56 (1)	10.00				
G- F	0 / 0	-96.5	-96.5	0.03 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
D	18-3-6	-414	-414	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 5-6-10
END SETBACK = 7-2-14
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: PARTIAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 10-10-0 OF SPAN MEASURED FROM THE RIGHT.

GIRDER TYPE: CStdGIrder
START DISTANCE = 0-0
START SPAN CARRIED = 25-5-1
END DISTANCE = 23-10-0
END SPAN CARRIED = 25-5-1
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.79")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.79")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.26")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.57/1.00 (D-E:1) , BC=0.86/1.00 (H-J:1) , WB=0.64/1.00 (E-H:1) , SSI=0.72/1.00 (H-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

A-18082082

CONTINUED ON PAGE 2

PLATES (table is in inches)

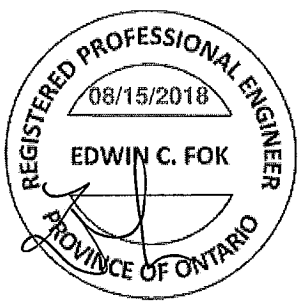
JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	10.0	2.00	5.00
B	TTWW+m	MT20	8.0	9.0	4.00	2.25
C	TMW+w	MT20	2.0	4.0		
D	TTWW+m	MT20	8.0	9.0	4.00	2.25
E	TMVW-t	MT20	5.0	10.0	2.00	5.00
G	BMV1+t	MT20	6.0	9.0	Edge	0.50
H	BMWW-t	MT20	7.0	8.0	4.25	3.00
I	BS-t	MT20	6.0	9.0		
J	BMWWW-t	MT20	8.0	9.0	4.25	4.50
K	BMWW-t	MT20	7.0	8.0	4.25	3.00
L	BMV1+t	MT20	6.0	9.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 414.1 lbs FACTORED DOWN AT 18-3-6 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

Note: Lateral brace(s) shown shall be 2x4 SPF#2.



TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (A) (INPUT = 0.90)
JSI METAL= 0.94 (D) (INPUT = 1.00)

A: 18 of 20 of 2 (2)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292594	H10T	5	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Sat Feb 17 13:44:50 2018 Page 1
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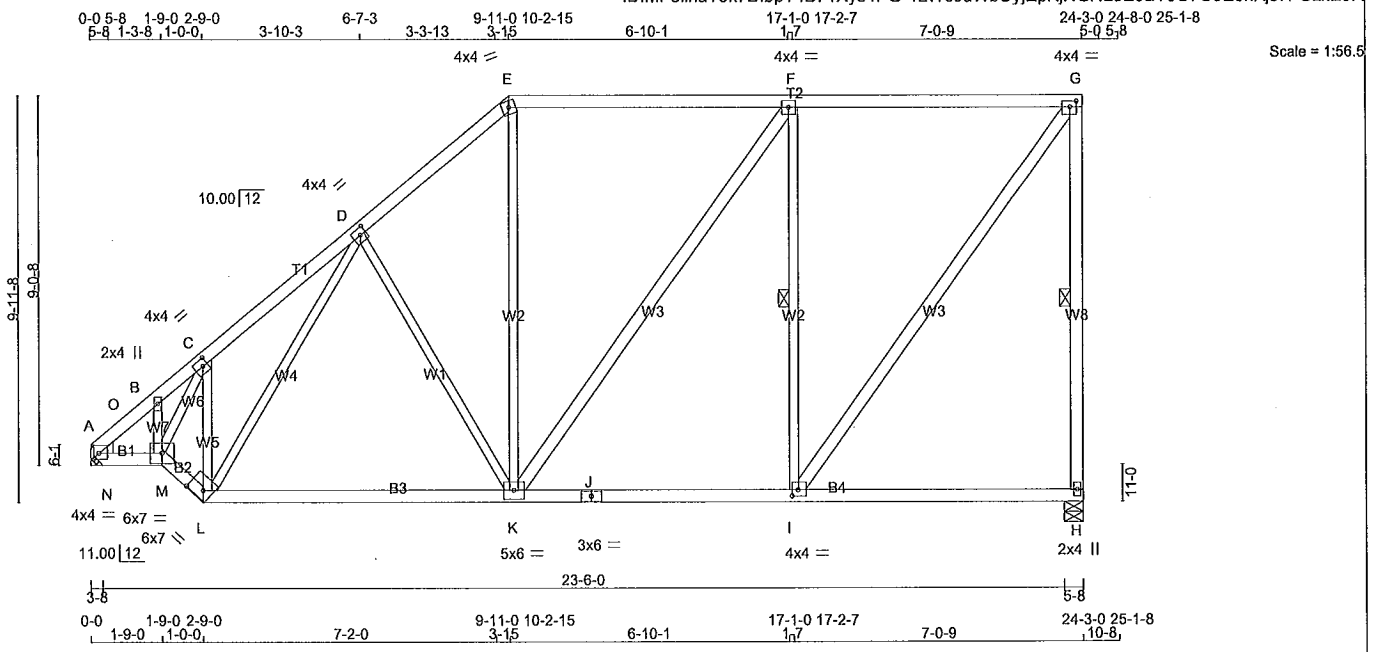

TOTAL WEIGHT = 5 X 130 = 649 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
A - M	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
	2x4	DRY	No.2	SPF
	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBH1-l	MT20	4.0	4.0	1.75	1.50
B	TMW+w	MT20	2.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TMWW-t	MT20	4.0	4.0	2.00	1.75
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMWW-t	MT20	4.0	4.0	1.75	1.75
H	BMW1+w	MT20	2.0	4.0		
I	BMWW-t	MT20	4.0	4.0	1.75	1.75
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	5.0	6.0		
L	BBWW-h	MT20	6.0	7.0	2.25	4.50
M	BBWW-l	MT20	6.0	7.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

REGISTERED PROFESSIONAL ENGINEER
02/17/2018
EDWIN C. FOK
PROVINCE OF ONTARIO

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG	HEEL WEDGE
H	1237	0	1237	0	0	5-8	1-13	
A	1193	0	1193	0	0	HANGER BY OTHERS		2x4 L
						MIN. SEAT SIZE: 1-8		

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	888	506 / 0	0 / 0	0 / 0	0 / 0	382 / 0	0 / 0
A	853	506 / 0	0 / 0	0 / 0	0 / 0	347 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

FOR SECTION E-G, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.04 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-H, F-I.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- O	-1682 / 0	-78.0	-78.0	0.11 (1)	5.04
O- B	-1581 / 0	-78.0	-78.0	0.12 (1)	5.15
B- C	-1441 / 0	-78.0	-78.0	0.12 (1)	5.34
C- D	-1151 / 0	-78.0	-78.0	0.14 (1)	5.79
D- E	-997 / 0	-78.0	-78.0	0.13 (1)	6.12
E- F	-751 / 0	-85.5	-85.5	0.54 (1)	2.00
F- G	-687 / 0	-85.5	-85.5	0.54 (1)	2.00
A- N	0 / 1143	-18.5	-18.5	0.22 (1)	10.00
N- M	0 / 1143	-18.5	-18.5	0.22 (1)	10.00
M- L	0 / 1179	-18.5	-18.5	0.19 (1)	10.00
L- K	0 / 882	-18.5	-18.5	0.32 (4)	10.00
K- J	0 / 687	-18.5	-18.5	0.31 (4)	10.00
J- I	0 / 687	-18.5	-18.5	0.31 (4)	10.00
I- H	0 / 0	-18.5	-18.5	0.19 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- B	0 / 149	0.03 (1)
M- C	0 / 734	0.17 (1)
L- C	-743 / 0	0.17 (1)
H- G	-1185 / 0	0.57 (1)
K- E	0 / 290	0.07 (1)
K- F	0 / 111	0.02 (1)
I- F	-827 / 0	0.56 (1)
I- G	0 / 1183	0.19 (1)
L- D	0 / 18	0.01 (4)
D- K	-271 / 0	0.31 (1)
N- O	0 / 82	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.80")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.80")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.54/1.00 (E-F:1) , BC=0.32/1.00 (K-L:4) , WB=0.57/1.00 (G-H:1) , SSI=0.29/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (A) (INPUT = 0.90)
JSI METAL= 0.39 (A) (INPUT = 1.00)

A-18023521

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292593	H31G	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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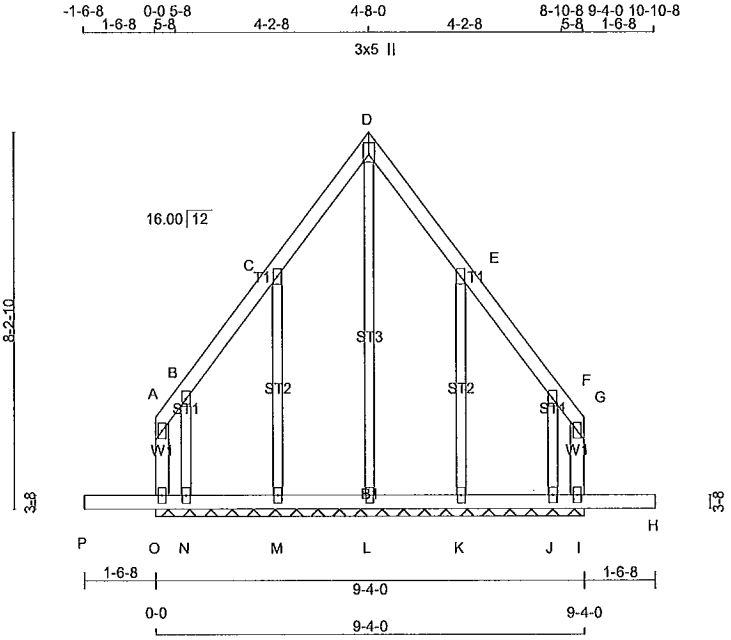


TOTAL WEIGHT = 54 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
O - A	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
P - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

SEASONED LUMBER.

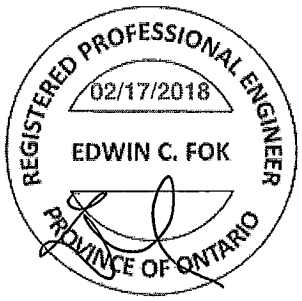
- STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0		
B, C, E, F						
B	TMW+w	MT20	2.0	4.0		
D	TTW+p	MT20	3.0	5.0	2.00	Edge
G	TMV+p	MT20	2.0	4.0		
I	BMV1+p	MT20	2.0	4.0		
J, K, L, M, N						
J	BMW1+w	MT20	2.0	4.0		
O	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

PROVIDE ANCHORAGE AT BEARING JOINT N FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT J FOR 150 LBS FACTORED UPLIFT
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- A	-20 / 0	0.0	0.0	0.02 (1)	7.81	L- D	-178 / 0	0.22 (1)
A- B	0 / 5	-78.0	-78.0	0.02 (1)	10.00	M- C	-165 / 0	0.07 (1)
B- C	0 / 10	-78.0	-78.0	0.04 (1)	10.00	N- B	-90 / 0	0.02 (1)
C- D	0 / 9	-78.0	-78.0	0.04 (1)	10.00	K- E	-165 / 0	0.07 (1)
D- E	0 / 9	-78.0	-78.0	0.04 (1)	10.00	J- F	-90 / 0	0.02 (1)
E- F	0 / 10	-78.0	-78.0	0.04 (1)	10.00			
F- G	0 / 5	-78.0	-78.0	0.02 (1)	10.00			
I- G	-20 / 0	0.0	0.0	0.02 (1)	7.81			
P- O	0 / 0	-96.5	-96.5	0.16 (1)	10.00			
O- N	-3 / 2	-18.5	-18.5	0.14 (1)	10.00			
N- M	-4 / 0	-18.5	-18.5	0.02 (4)	10.00			
M- L	-6 / 0	-18.5	-18.5	0.02 (4)	10.00			
L- K	-6 / 0	-18.5	-18.5	0.02 (4)	10.00			
K- J	-4 / 0	-18.5	-18.5	0.02 (4)	10.00			
J- I	-3 / 2	-18.5	-18.5	0.14 (1)	10.00			
I- H	0 / 0	-96.5	-96.5	0.16 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.04/1.00 (B-C:1) , BC=0.16/1.00 (H-I:1) , WB=0.22/1.00 (D-L:1) , SSI=0.14/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (D) (INPUT = 0.90)
JSI METAL= 0.08 (E) (INPUT = 1.00)

A-18023520

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292593	H31	3	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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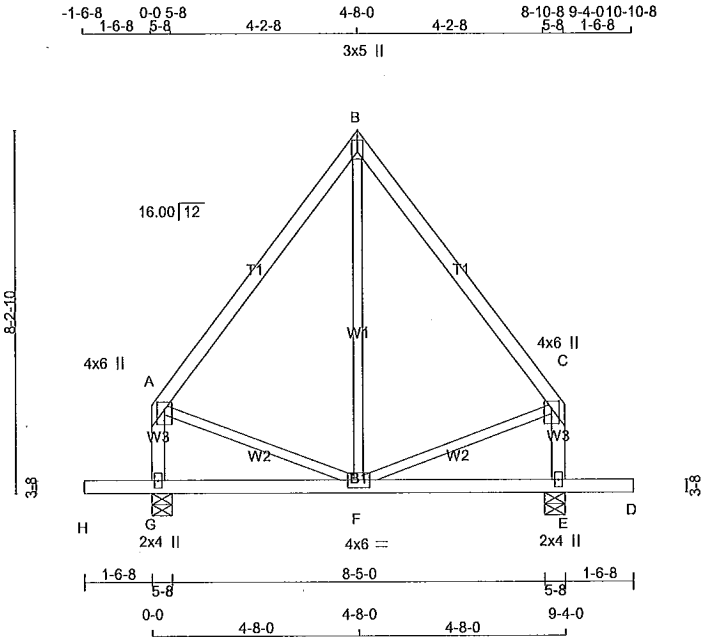


TOTAL WEIGHT = 3 X 50 = 150 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
G - A	2x4	DRY	No.2	SPF
E - C	2x4	DRY	No.2	SPF
H - D	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

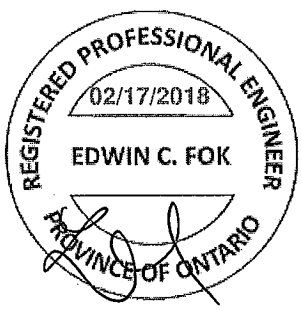
SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.00	2.00
B	TTW+p	MT20	3.0	5.0	2.00	Edge
C	TMVW+p	MT20	4.0	6.0	2.00	2.00
E	BMV1+p	MT20	2.0	4.0		
F	BMWWW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	599	0	599	0	0	5-8	1-8
E	599	0	599	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	427	261 / 0	0 / 0	0 / 0	0 / 0	166 / 0	0 / 0
E	427	261 / 0	0 / 0	0 / 0	0 / 0	166 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-234 / 0	-78.0	-78.0	1	0.22 (1)	6.25	F- B	0 / 79	0.03 (4)
B- C	-234 / 0	-78.0	-78.0	1	0.22 (1)	6.25	A- F	0 / 148	0.03 (1)
G- A	-417 / 0	0.0	0.0	1	0.05 (1)	7.81	F- C	0 / 148	0.03 (1)
E- C	-417 / 0	0.0	0.0	1	0.05 (1)	7.81			
H- G	0 / 0	-96.5	-96.5	1	0.16 (1)	10.00			
G- F	0 / 0	-18.5	-18.5	1	0.11 (4)	10.00			
F- E	0 / 0	-18.5	-18.5	1	0.11 (4)	10.00			
E- D	0 / 0	-96.5	-96.5	1	0.16 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.31")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.31")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.22/1.00 (B-C:1) , BC=0.16/1.00 (D-E:1) , WB=0.03/1.00 (C-F:1) , SSI=0.12/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (A) (INPUT = 0.90)
JSI METAL= 0.12 (C) (INPUT = 1.00)

A-18023519

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292593	H30	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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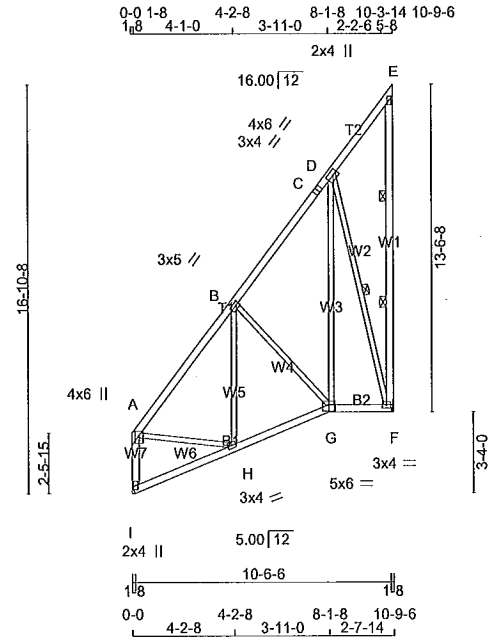


Scale: 1/8"=1'

TOTAL WEIGHT = 2 X 79 = 159 lb

[M][F]

LUMBER

N. L. G. A. RULES

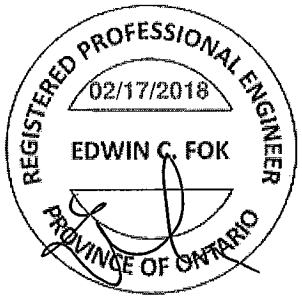
CHORDS	SIZE		LUMBER	DESCR.
I - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.00	2.00
B	TMWW-t	MT20	3.0	5.0	1.50	1.50
C	TS-t	MT20	3.0	4.0		
D	TMWW-t	MT20	4.0	6.0	2.00	2.50
E	TMV+p	MT20	2.0	4.0		
F	BMVW1-t	MT20	3.0	4.0		
G	BBWW-I	MT20	5.0	6.0	3.00	3.00
H	BMWW-t	MT20	3.0	4.0		
I	BMV1+p	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
I	520	0	520	0	0	IN-SX	IN-SX
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	
F	520	0	520	0	0	HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	371	226 / 0	0 / 0	0 / 0	0 / 0	144 / 0	0 / 0
F	371	226 / 0	0 / 0	0 / 0	0 / 0	144 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2 LATERAL BRACE(S) AT 1/ 3 LENGTH OF E-F.
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- A	-489 / 0	0.0	0.0	0.07 (1)	7.81	G- D	0 / 310	0.07 (1)
A- B	-393 / 0	-78.0	-78.0	0.16 (1)	6.25	D- F	-440 / 0	0.33 (1)
B- C	-205 / 0	-78.0	-78.0	0.15 (1)	6.25	A- H	0 / 253	0.06 (1)
C- D	-205 / 0	-78.0	-78.0	0.15 (1)	6.25	H- B	-36 / 54	0.02 (1)
D- E	-23 / 0	-78.0	-78.0	0.08 (1)	6.25	B- G	-206 / 0	0.13 (1)
F- E	-70 / 0	0.0	0.0	0.03 (1)	6.25			
I- H	0 / 4	-18.5	-18.5	0.09 (4)	10.00			
H- G	0 / 269	-18.5	-18.5	0.11 (4)	10.00			
G- F	0 / 111	-18.5	-18.5	0.05 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	21.0	PSF	
	DL =	6.0	PSF	
BOT CH.	LL =	0.0	PSF	
	DL =	7.4	PSF	
TOTAL LOAD	=	34.4	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.36")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.36")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.16/1.00 (A-B:1) , BC=0.11/1.00 (G-H:4) , WB=0.33/1.00 (D-F:1) , SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.68 (F) (INPUT = 0.90)
JSI METAL= 0.18 (I) (INPUT = 1.00)

A-18023518

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292593	H29	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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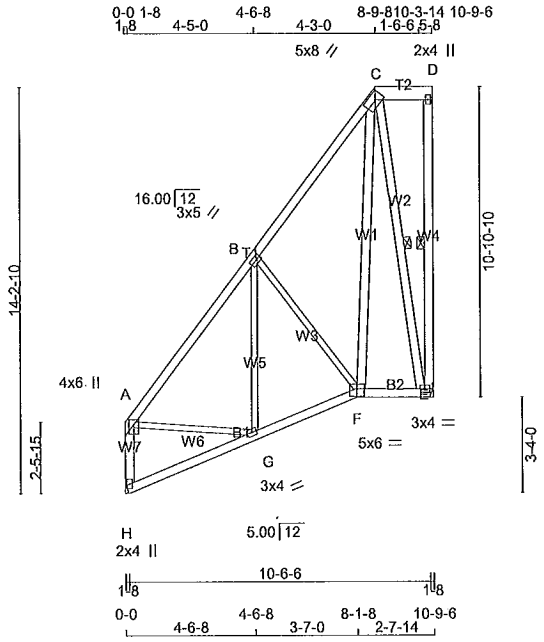


Scale = 1:80.7

TOTAL WEIGHT = 84 lb

[M][F]

LUMBER
N. L. G. A. RULES

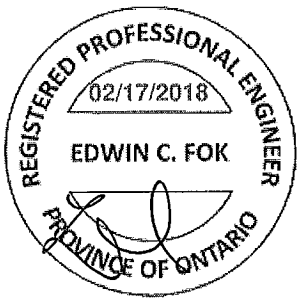
CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x6	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
[illegible]	2x4	DRY	No.2	SPF
[illegible]	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.00	2.00
B	TMWW-t	MT20	3.0	5.0	1.50	1.50
C	TTWW-h	MT20	5.0	8.0	2.50	3.25
D	TMV+p	MT20	2.0	4.0		
E	BMVW1-t	MT20	3.0	4.0		
F	BBWW-t	MT20	5.0	6.0	3.00	3.00
G	BMWW-t	MT20	3.0	4.0		
H	BMV1+p	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
E	520	0	520	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
H	520	0	520	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	371	226 / 0	0 / 0	0 / 0	0 / 0	144 / 0	0 / 0
H	371	226 / 0	0 / 0	0 / 0	0 / 0	144 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-E, C-E.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-387 / 0	-78.0	-78.0	0.20 (1)	6.25	A- G	0 / 252	0.06 (1)
B- C	-205 / 0	-78.0	-78.0	0.19 (1)	6.25	G- B	-26 / 59	0.02 (4)
C- D	0 / 0	-78.0	-78.0	0.03 (1)	10.00	B- F	-251 / 0	0.16 (1)
E- D	-78 / 0	0.0	0.0	0.05 (1)	6.25	F- C	0 / 355	0.06 (1)
H- A	-486 / 0	0.0	0.0	0.07 (1)	7.81	C- E	-426 / 0	0.26 (1)
H- G	0 / 4	-18.5	-18.5	0.09 (4)	10.00			
G- F	0 / 269	-18.5	-18.5	0.11 (4)	10.00			
F- E	0 / 79	-18.5	-18.5	0.05 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.36")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.36")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.20/1.00 (A-B:1) , BC=0.11/1.00 (F-G:4) , WB=0.26/1.00 (C-E:1) , SSI=0.10/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.46 (E) (INPUT = 0.90)
JSI METAL= 0.17 (H) (INPUT = 1.00)

A-18023517

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292593	H28	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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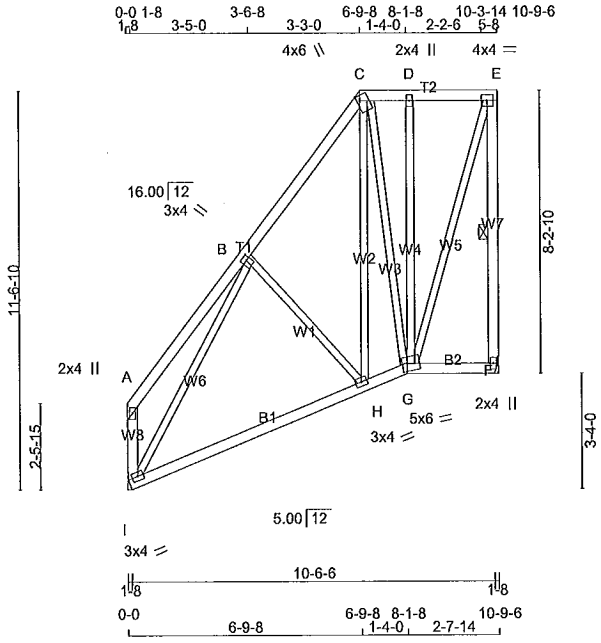


Scale = 1:67.0

TOTAL WEIGHT = 78 lb

[M][F]

LUMBER

N. L. G. A. RULES

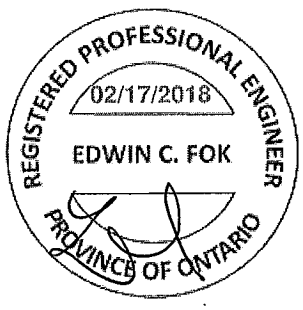
CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - A	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

EASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0		
B	TMWW+t	MT20	3.0	4.0	1.50	1.25
C	TTWW+m	MT20	4.0	6.0	1.75	1.00
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	4.0		
F	BMV1+p	MT20	2.0	4.0		
G	BBWWW-m	MT20	5.0	6.0	2.75	1.75
H	BMWW-t	MT20	3.0	4.0		
I	BMVW1-t	MT20	3.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	520	0	520	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
I	520	0	520	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	371	226 / 0	0 / 0	0 / 0	0 / 0	144 / 0	0 / 0
I	371	226 / 0	0 / 0	0 / 0	0 / 0	144 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 25	-78.0	-78.0	0.15 (1)	10.00	H- C	0 / 261	0.07 (4)
B- C	-297 / 0	-78.0	-78.0	0.11 (1)	6.25	C- G	-185 / 0	0.25 (1)
C- D	-134 / 0	-78.0	-78.0	0.03 (1)	6.25	G- D	-186 / 0	0.25 (1)
D- E	-137 / 0	-78.0	-78.0	0.06 (1)	6.25	G- E	0 / 431	0.10 (1)
F- E	-496 / 0	0.0	0.0	0.15 (1)	6.25	I- B	-522 / 0	0.55 (1)
I- A	-101 / 0	0.0	0.0	0.01 (1)	7.81	B- H	-109 / 0	0.05 (1)
I- H	0 / 261	-18.5	-18.5	0.22 (4)	10.00			
H- G	0 / 155	-18.5	-18.5	0.21 (4)	10.00			
G- F	0 / 0	-18.5	-18.5	0.04 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.36")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.36")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.15/1.00 (E-F:1) , BC=0.22/1.00 (H-I:4) , WB=0.55/1.00 (B-I:1) , SSI=0.10/1.00 (G-H:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (B) (INPUT = 0.90)
JSI METAL= 0.32 (B) (INPUT = 1.00)

A-18023516

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292593	H27	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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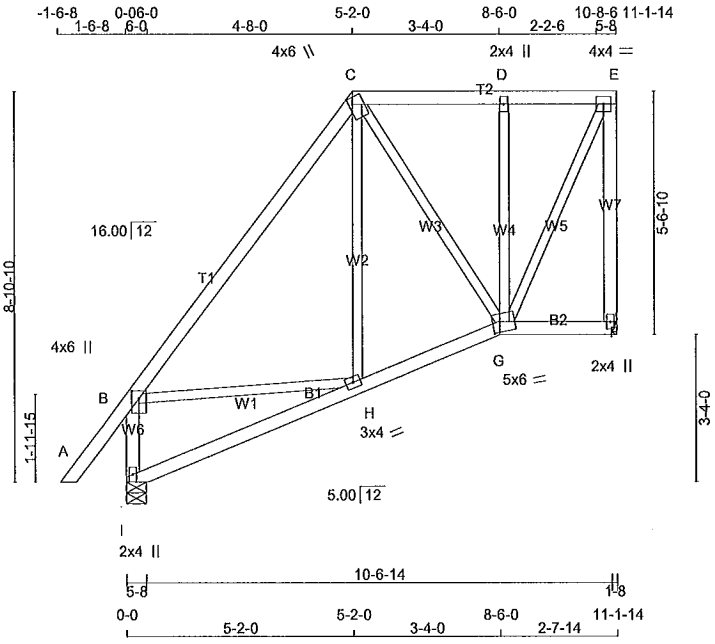

Scale = 1:52.5

TOTAL WEIGHT = 63 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TTWW+m	MT20	4.0	6.0	1.75	1.00
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	4.0		
F	BMV1+p	MT20	2.0	4.0		
G	BBWWW-m	MT20	5.0	6.0	2.75	2.00
H	BMWW-t	MT20	3.0	4.0		
I	BMV1+p	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

REGISTERED PROFESSIONAL ENGINEER
02/17/2018
EDWIN C. FOK
PROVINCE OF ONTARIO

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	538	0	538	0	0	IN-SX	IN-SX
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	
I	646	0	646	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	384	234 / 0	0 / 0	0 / 0	0 / 0	150 / 0	0 / 0
I	458	292 / 0	0 / 0	0 / 0	0 / 0	166 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 43	-78.0	-78.0	0.11 (1)		10.00	H- C	0 / 81	0.03 (4)
B- C	-392 / 0	-78.0	-78.0	0.38 (1)		6.25	C- G	-27 / 0	0.02 (4)
C- D	-220 / 0	-78.0	-78.0	0.11 (1)		6.25	G- D	-286 / 0	0.13 (1)
D- E	-226 / 0	-78.0	-78.0	0.11 (1)		6.25	G- E	0 / 500	0.11 (1)
F- E	-514 / 0	0.0	0.0	0.28 (1)		7.81	B- H	0 / 235	0.05 (1)
I- B	-606 / 0	0.0	0.0	0.07 (1)		7.81			
I- H	0 / 4	-18.5	-18.5	0.11 (4)		10.00			
H- G	0 / 248	-18.5	-18.5	0.13 (4)		10.00			
G- F	0 / 0	-18.5	-18.5	0.04 (4)		10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.37")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.37")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.38/1.00 (B-C:1) , BC=0.13/1.00 (G-H:4) , WB=0.13/1.00 (D-G:1) , SSI=0.12/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.62 (E) (INPUT = 0.90)
JSI METAL= 0.22 (I) (INPUT = 1.00)

A-18023515

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292593	H26	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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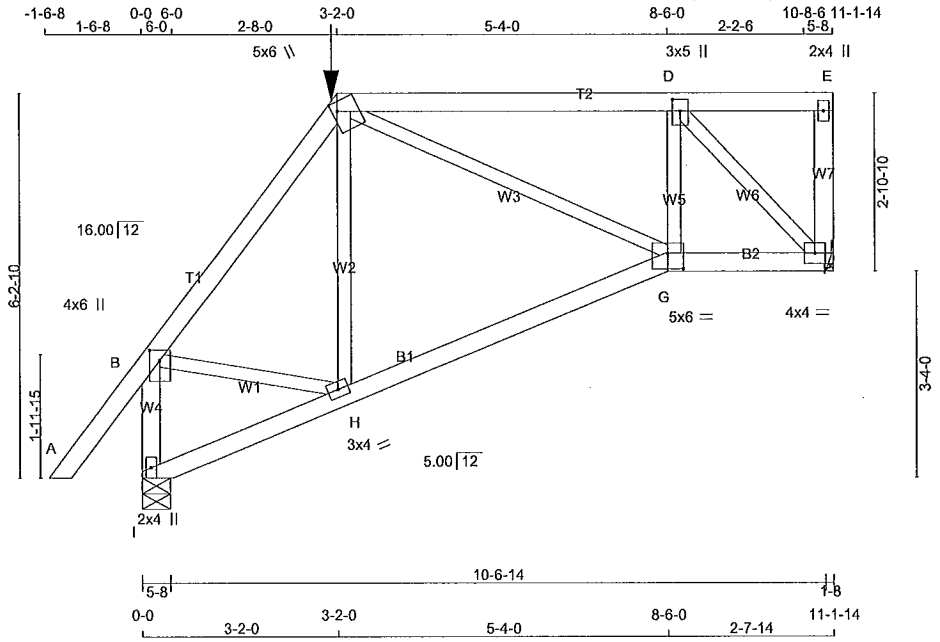


Scale = 1:37.3

TOTAL WEIGHT = 51 lb

(M)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

(EASONED LUMBER.

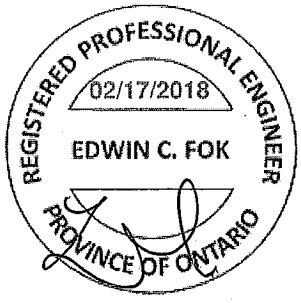
PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TTWW+m	MT20	5.0	6.0	1.75	1.00
D	TMWW+t	MT20	3.0	5.0	2.25	1.50
E	TMV+p	MT20	2.0	4.0		
F	BMVW1-t	MT20	4.0	4.0		
G	BBWW-l	MT20	5.0	6.0	3.00	3.00
H	BMWW-t	MT20	3.0	4.0		
I	BMV1+p	MT20	2.0	4.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 68.6 lbs FACTORED DOWN AT 3-2-0 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	704	0	704	0	0	IN-SX	IN-SX
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	
I	790	0	790	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	502	307 / 0	0 / 0	0 / 0	0 / 0	195 / 0	0 / 0
I	561	354 / 0	0 / 0	0 / 0	0 / 0	208 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 43	-78.0	-78.0	0.12 (1)	10.00	H- C	-68 / 66	0.03 (1)
B- C	-609 / 0	-78.0	-78.0	0.16 (1)	6.25	C- G	0 / 312	0.08 (1)
C- D	-642 / 0	-100.7	-100.7	0.42 (1)	6.25	G- D	0 / 115	0.04 (4)
D- E	0 / 0	-100.7	-100.7	0.40 (1)	10.00	D- F	-911 / 0	0.22 (1)
F- E	-25 / 0	0.0	0.0	0.00 (1)	7.81	B- H	0 / 366	0.09 (1)
I- B	-769 / 0	0.0	0.0	0.10 (1)	7.81			
I- H	0 / 8	-23.9	-23.9	0.16 (4)	10.00			
H- G	0 / 387	-23.9	-23.9	0.20 (4)	10.00			
G- F	0 / 639	-23.9	-23.9	0.14 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	3-2-0	-69	-69	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	21.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 3-2-0
RIGHT SETBACK = 0-0
END SETBACK = 3-2-0
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.37")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.37")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.42/1.00 (C-D:1) , BC=0.20/1.00 (G-H:4) , WB=0.22/1.00 (D-F:1) , SSI=0.28/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (D) (INPUT = 0.90)
JSI METAL= 0.38 (D) (INPUT = 1.00)

A-18023514

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292593	H25T	5	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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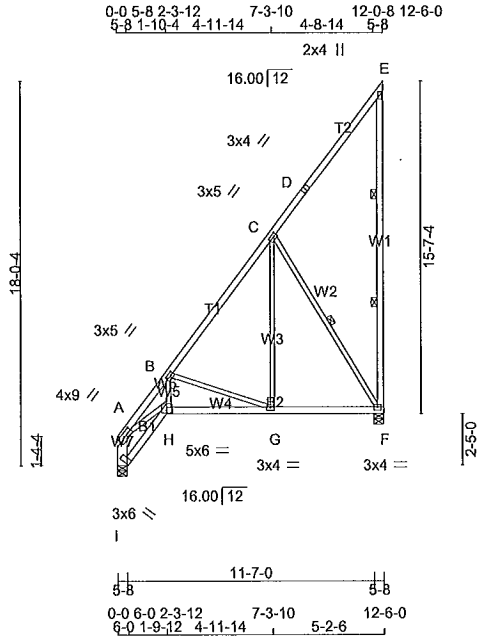


Scale = 1:108.2

TOTAL WEIGHT = 5 X 81 = 404 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - A	2x6	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

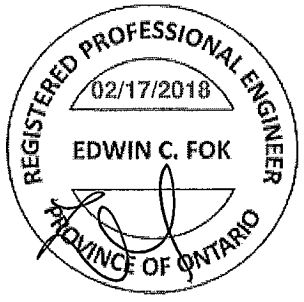
EASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	9.0	2.00	3.00
B	TMWW-t	MT20	3.0	5.0	1.50	1.50
C	TMWW-t	MT20	3.0	5.0	1.50	1.50
D	TS-t	MT20	3.0	4.0		
E	TMV+p	MT20	2.0	4.0		
F	BMVW1-t	MT20	3.0	4.0		
G	BMWW-t	MT20	3.0	4.0		
H	BBWW-I	MT20	5.0	6.0	3.00	3.00
I	BVM1+I	MT20	3.0	6.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	603	0	603	0	0	5-8	1-8
I	603	0	603	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	430	262 / 0	0 / 0	0 / 0	0 / 0	168 / 0	0 / 0
I	430	262 / 0	0 / 0	0 / 0	0 / 0	168 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, I
BEARING SIZE FACTOR = 1.15 AT JNT(S) I (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.68 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2 LATERAL BRACE(S) AT 1/ 3 LENGTH OF E-F.
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-1237 / 0	-78.0	-78.0	0.10 (1)	5.68	H- B	0 / 556	0.13 (1)
B- C	-400 / 0	-78.0	-78.0	0.25 (1)	6.25	B- G	-499 / 0	0.23 (1)
C- D	-35 / 0	-78.0	-78.0	0.26 (1)	6.25	G- C	0 / 279	0.07 (4)
D- E	-35 / 0	-78.0	-78.0	0.26 (1)	6.25	C- F	-480 / 0	0.33 (1)
F- E	-159 / 0	0.0	0.0	0.09 (1)	6.25	A- H	0 / 903	0.20 (1)
I- A	-582 / 0	0.0	0.0	0.04 (1)	7.81			
I- H	0 / 0	-18.5	-18.5	0.03 (4)	10.00			
H- G	0 / 724	-18.5	-18.5	0.20 (4)	10.00			
G- F	0 / 255	-18.5	-18.5	0.16 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.42")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.42")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.26/1.00 (C-E:1) , BC=0.20/1.00 (G-H:4) , WB=0.33/1.00 (C-F:1) , SSI=0.12/1.00 (C-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.71 (G) (INPUT = 0.90)
JSI METAL= 0.38 (H) (INPUT = 1.00)

A-18023513

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292593	H24T	2	1	TRUSS DESC.	

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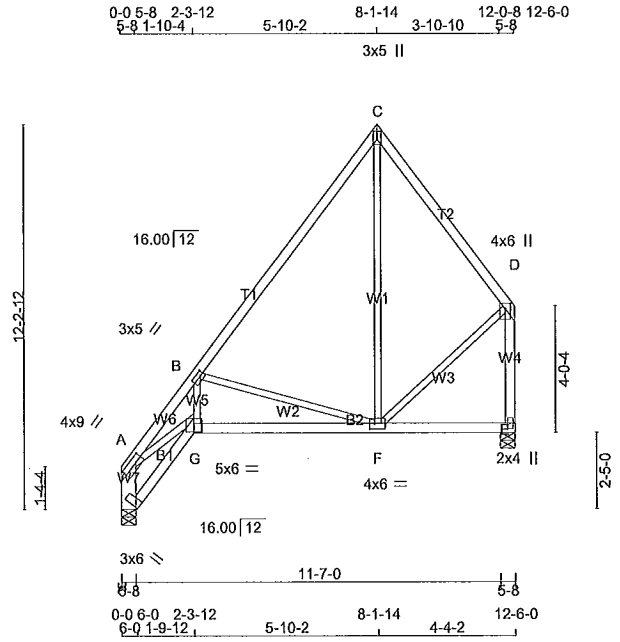


TOTAL WEIGHT = 2 X 67 = 133 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
H - A	2x6	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

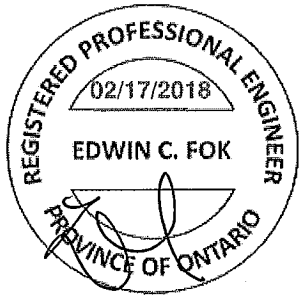
EASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	9.0	2.00	3.00
B	TMWW-t	MT20	3.0	5.0	1.50	1.50
C	TTW+p	MT20	3.0	5.0	2.00	Edge
D	TMVW+p	MT20	4.0	6.0	2.00	2.00
E	BMV1+p	MT20	2.0	4.0		
F	BMWWW-t	MT20	4.0	6.0	2.00	2.00
G	BBWW-I	MT20	5.0	6.0	3.00	3.00
H	BVM1+I	MT20	3.0	6.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H	603	0	603	0	0	5-8	1-8
E	603	0	603	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	430	262 / 0	0 / 0	0 / 0	0 / 0	167 / 0	0 / 0
E	430	262 / 0	0 / 0	0 / 0	0 / 0	168 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, E
BEARING SIZE FACTOR = 1.15 AT JNT(S) H (BASED ON SUPPORT DEPTH = 1-8)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.33 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-1334 / 0	-78.0	-78.0	0.25 (1)	5.33	G- B	0 / 620	0.14 (1)
B- C	-344 / 0	-78.0	-78.0	0.33 (1)	6.25	B- F	-648 / 0	0.41 (1)
C- D	-313 / 0	-78.0	-78.0	0.19 (1)	6.25	F- C	0 / 146	0.05 (4)
H- A	-582 / 0	0.0	0.0	0.04 (1)	7.81	F- D	0 / 245	0.06 (1)
E- D	-577 / 0	0.0	0.0	0.16 (1)	7.81	A- G	0 / 1006	0.23 (1)
H- G	0 / 0	-18.5	-18.5	0.03 (4)	10.00			
G- F	0 / 807	-18.5	-18.5	0.22 (4)	10.00			
F- E	0 / 0	-18.5	-18.5	0.15 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.42")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.42")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.33/1.00 (B-C:1) , BC=0.22/1.00 (F-G:4) , WB=0.41/1.00 (B-F:1) , SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (F) (INPUT = 0.90)
JSI METAL= 0.42 (G) (INPUT = 1.00)

A-18023512

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292593	H23P	4	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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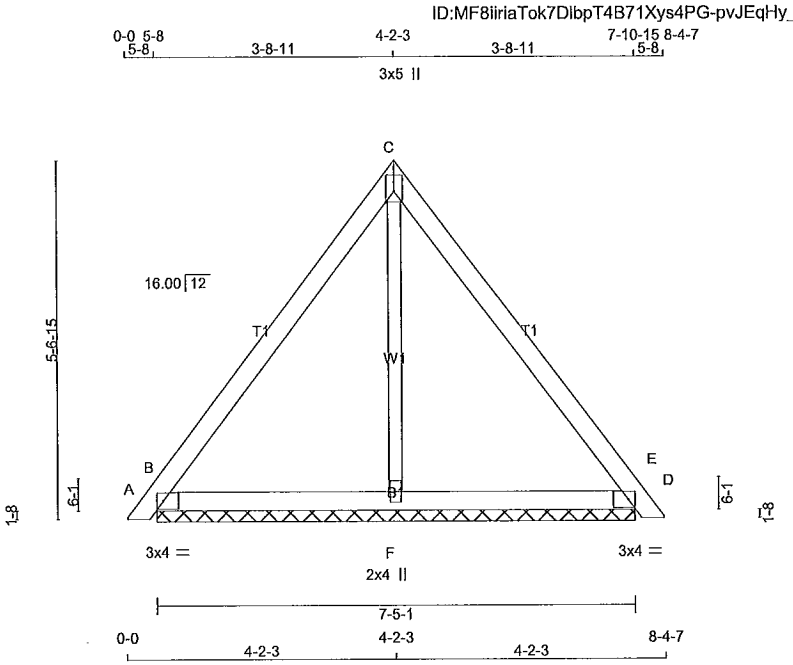


TOTAL WEIGHT = 4 X 28 = 113 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

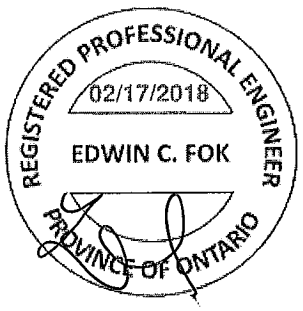
DRY: SEASONED LUMBER.

3 (table is in inches)

	PE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.75
C	TTW+p	MT20	3.0	5.0	2.00	Edge
D	TMB1-I	MT20	3.0	4.0	1.50	2.75
F	BMW1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
B	277	0	277	0	0	7-5-1	7-4
D	277	0	277	0	0	7-5-1	7-4
F	216	0	216	0	0	7-5-1	7-4

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	195	135 / 0	0 / 0	0 / 0	0 / 0	59 / 0	0 / 0
D	195	135 / 0	0 / 0	0 / 0	0 / 0	59 / 0	0 / 0
F	159	70 / 0	0 / 0	0 / 0	0 / 0	89 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)		LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM	TO			FR-TO			
A- B	0 / 10	-78.0	-78.0	0.01 (1)	10.00	F- C	-130 / 0	0.05 (1)	
B- C	-98 / 0	-78.0	-78.0	0.18 (1)	6.25				
C- D	-98 / 0	-78.0	-78.0	0.18 (1)	6.25				
D- E	0 / 10	-78.0	-78.0	0.01 (1)	10.00				
B- F	0 / 58	-18.5	-18.5	0.08 (4)	10.00				
F- D	0 / 58	-18.5	-18.5	0.08 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.18/1.00 (B-C:1) , BC=0.08/1.00 (B-F:4) , WB=0.05/1.00 (C-F:1) , SSI=0.07/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.33 (D) (INPUT = 0.90)
JSI METAL= 0.12 (D) (INPUT = 1.00)

A-18023511

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292593	H22P	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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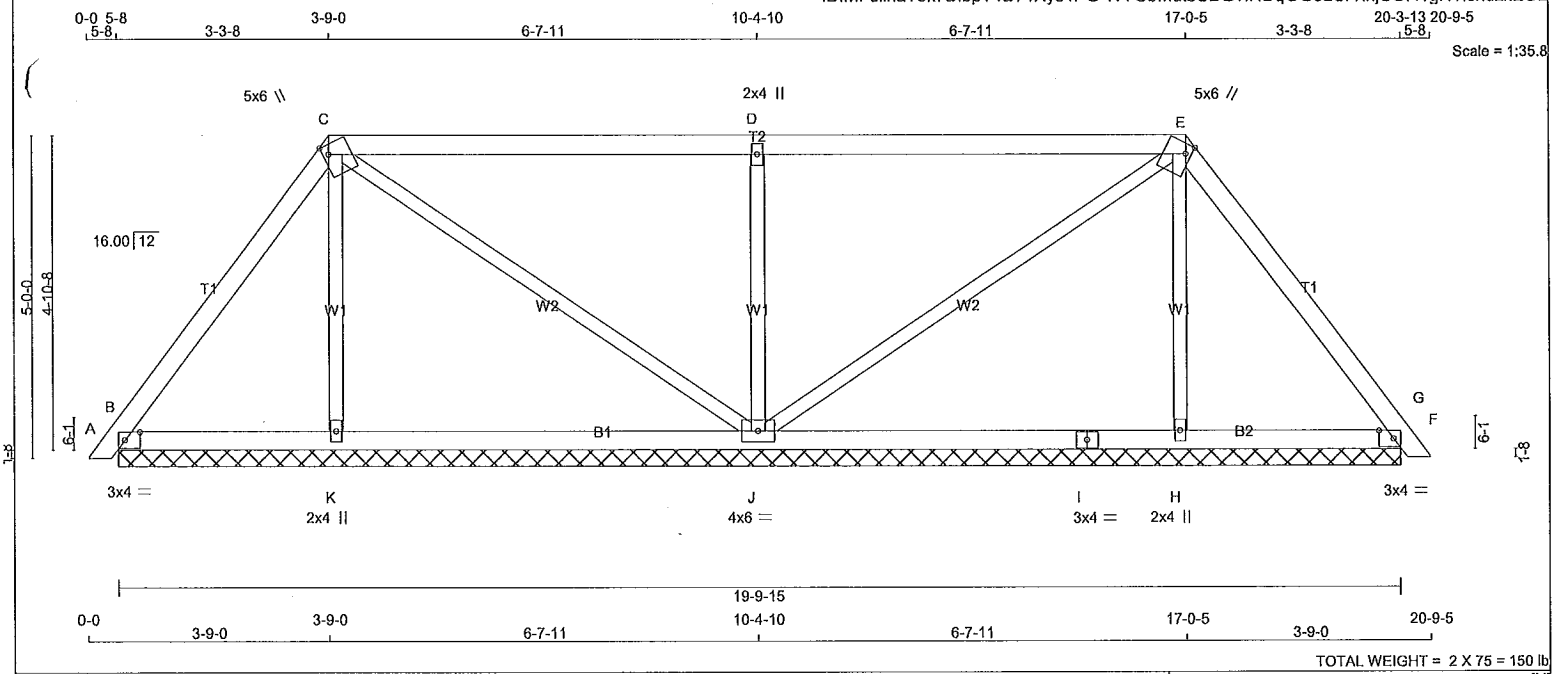


TOTAL WEIGHT = 2 X 75 = 150 lb

[M]

LUMBER

N. L. G. A. RULES

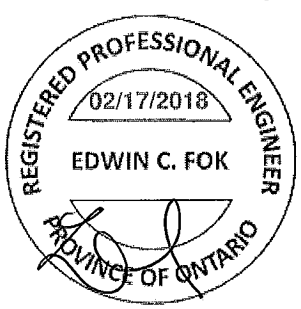
CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
B - I	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is In inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.75
C	TTWW+m	MT20	5.0	6.0	1.75	1.00
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	1.75	1.00
F	TMB1-I	MT20	3.0	4.0	1.50	2.75
H	BMW1+w	MT20	2.0	4.0		
I	BS-t	MT20	3.0	4.0		
J	BMWWW1-t	MT20	4.0	6.0		
K	BMW1+w	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	262	0	262	0	0	19-9-15 (4-9-15)	4-11
K	299	0	299	0	0	19-9-15 (4-9-15)	4-11
J	847	0	847	0	0	19-9-15 (4-9-15)	4-11
H	299	0	299	0	0	19-9-15 (4-9-15)	4-11
F	262	0	262	0	0	19-9-15 (4-9-15)	4-11

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	183	132 / 0	0 / 0	0 / 0	0 / 0	51 / 0	0 / 0
K	218	105 / 0	0 / 0	0 / 0	0 / 0	112 / 0	0 / 0
J	600	387 / 0	0 / 0	0 / 0	0 / 0	213 / 0	0 / 0
H	218	105 / 0	0 / 0	0 / 0	0 / 0	112 / 0	0 / 0
F	183	132 / 0	0 / 0	0 / 0	0 / 0	51 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, K, J, H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 10	-78.0	-78.0	0.01 (1)	10.00
B- C	-111 / 0	-78.0	-78.0	0.14 (1)	6.25
C- D	-7 / 0	-78.0	-78.0	0.58 (1)	10.00
D- E	-7 / 0	-78.0	-78.0	0.58 (1)	10.00
E- F	-111 / 0	-78.0	-78.0	0.14 (1)	6.25
F- G	0 / 10	-78.0	-78.0	0.01 (1)	10.00
B- K	0 / 65	-18.5	-18.5	0.12 (4)	10.00
K- J	0 / 61	-18.5	-18.5	0.18 (4)	10.00
J- I	0 / 61	-18.5	-18.5	0.18 (4)	10.00
I- H	0 / 61	-18.5	-18.5	0.18 (4)	10.00
H- F	0 / 65	-18.5	-18.5	0.12 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-196 / 0	0.07 (1)
C- J	-64 / 0	0.08 (1)
J- D	-646 / 0	0.23 (1)
J- E	-64 / 0	0.08 (1)
H- E	-196 / 0	0.07 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	21.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.58/1.00 (C-D:1) , BC=0.18/1.00 (H-J:4) , WB=0.23/1.00 (D-J:1) , SSI=0.25/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.65 (J) (INPUT = 0.90)
JSI METAL= 0.14 (D) (INPUT = 1.00)

A-18023510

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292593	H21P	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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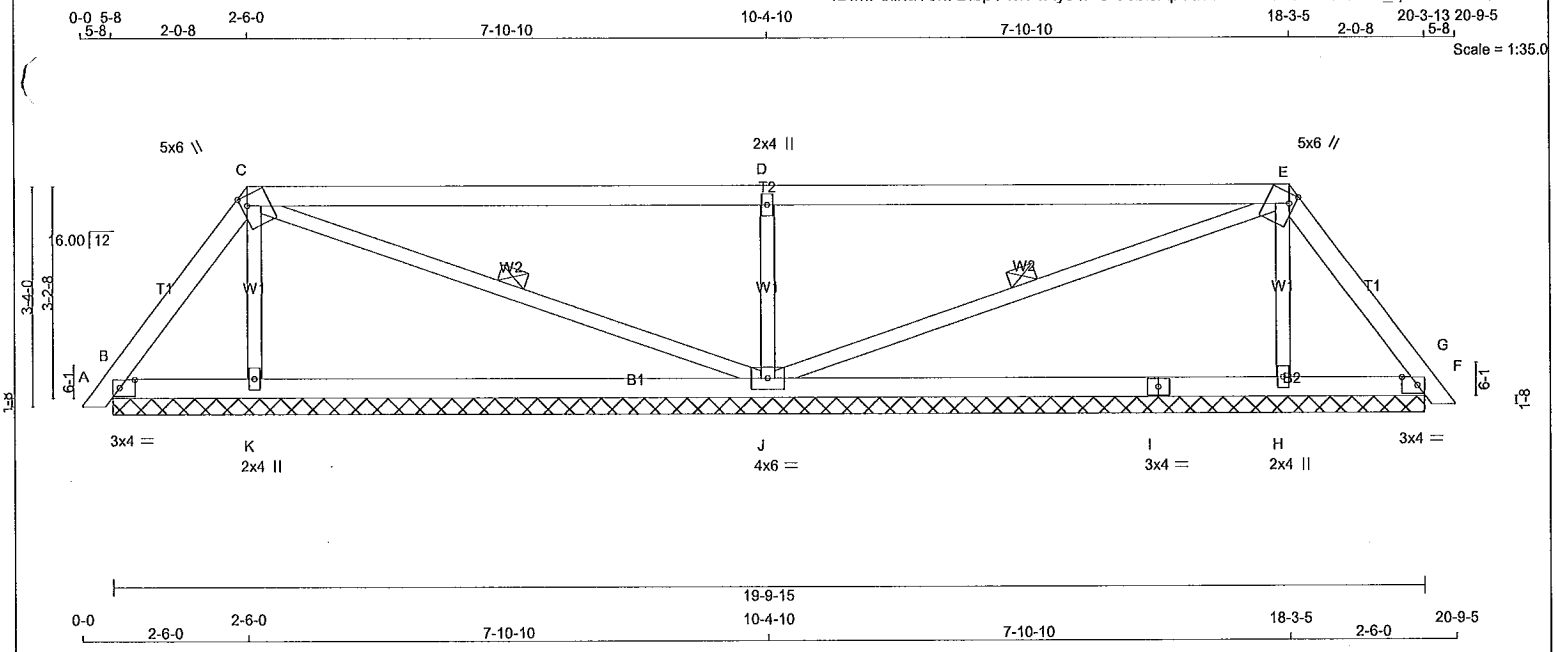


TOTAL WEIGHT = 2 X 70 = 139 lb

[M]

LUMBER

N. L. G. A. RULES

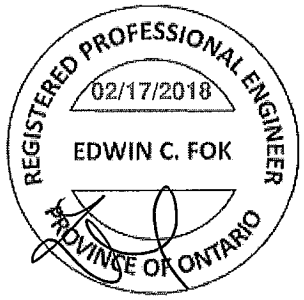
CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
B - I	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.75
C	TTWW+m	MT20	5.0	6.0	1.75	1.00
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	1.75	1.00
F	TMB1-I	MT20	3.0	4.0	1.50	2.75
H	BMW1+w	MT20	2.0	4.0		
I	BS-t	MT20	3.0	4.0		
J	BMWWW1-t	MT20	4.0	6.0		
K	BMW1+w	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	173	0	173	0	0	19-9-15 (4-0-0)0-4	
K	339	0	339	0	0	19-9-15 (4-0-0)0-4	
J	944	0	944	0	0	19-9-15 (4-0-0)0-4	
H	339	0	339	0	0	19-9-15 (4-0-0)0-4	
F	173	0	173	0	0	19-9-15 (4-0-0)0-4	

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	118	101 / 0	0 / 0	0 / 0	0 / 0	18 / 0	0 / 0
K	247	117 / 0	0 / 0	0 / 0	0 / 0	130 / 0	0 / 0
J	670	426 / 0	0 / 0	0 / 0	0 / 0	244 / 0	0 / 0
H	247	117 / 0	0 / 0	0 / 0	0 / 0	130 / 0	0 / 0
F	118	101 / 0	0 / 0	0 / 0	0 / 0	18 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, K, J, H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-J, E-J.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 10	-78.0	-78.0	0.01 (1)	10.00
B- C	-98 / 0	-78.0	-78.0	0.05 (1)	6.25
C- D	-16 / 0	-78.0	-78.0	0.83 (1)	6.25
D- E	-16 / 0	-78.0	-78.0	0.83 (1)	6.25
E- F	-98 / 0	-78.0	-78.0	0.05 (1)	6.25
F- G	0 / 10	-78.0	-78.0	0.01 (1)	10.00
B- K	0 / 57	-18.5	-18.5	0.17 (4)	10.00
K- J	0 / 49	-18.5	-18.5	0.25 (4)	10.00
J- I	0 / 49	-18.5	-18.5	0.25 (4)	10.00
I- H	0 / 49	-18.5	-18.5	0.25 (4)	10.00
H- F	0 / 57	-18.5	-18.5	0.17 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-218 / 0	0.04 (1)
C- J	-34 / 0	0.02 (1)
J- D	-768 / 0	0.14 (1)
J- E	-34 / 0	0.02 (1)
H- E	-218 / 0	0.04 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.83/1.00 (D-E:1) , BC=0.25/1.00 (H-J:4) , WB=0.14/1.00 (D-J:1) , SSI=0.30/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (J) (INPUT = 0.90)
JSI METAL= 0.16 (D) (INPUT = 1.00)

A-18023509

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292593	H20A	1	2	TRUSS DESC.	

Alpa Roof Truss, Maple

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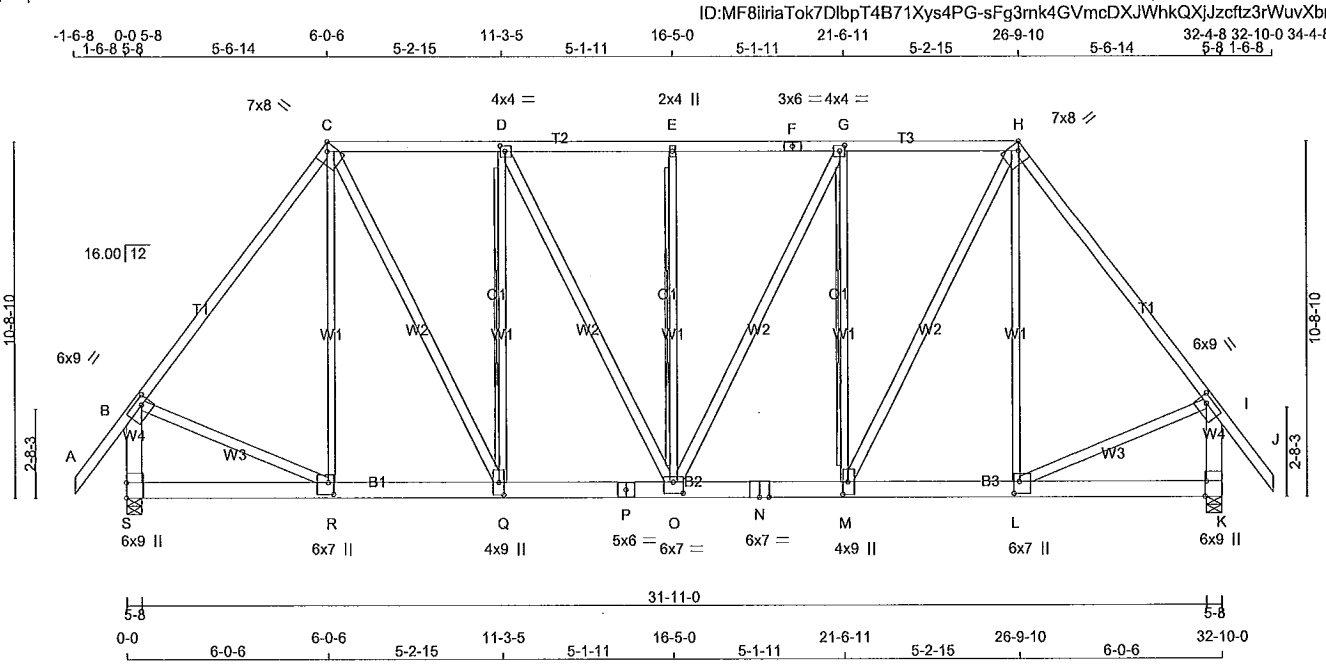


TOTAL WEIGHT = 2 X 226 = 451 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	2100F 1.8E	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	2100F 1.8E	SPF
S - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
S - P	2x6	DRY	1650F 1.5E	SPF
P - N	2x6	DRY	1650F 1.5E	SPF
N - K	2x6	DRY	1650F 1.5E	SPF
[illegible]BS	2x4	DRY	No.2	SPF
[illegible]T				
R - C	2x3	DRY	No.2	SPF
Q - D	2x3	DRY	No.2	SPF
O - E	2x3	DRY	No.2	SPF
M - G	2x3	DRY	No.2	SPF
L - H	2x3	DRY	No.2	SPF

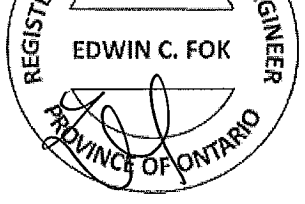
DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- F	1	12	TOP
F- H	1	12	TOP
H- J	1	12	TOP
S- B	2	12	TOP
K- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
S- P	2	9	SIDE(278.9)
P- N	2	9	SIDE(278.9)
N- K	2	9	SIDE(278.9)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	10869	0	10869	0	0	5-8	5-8
K	10869	0	10869	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	7746	4743 / 0	0 / 0	0 / 0	0 / 0	3003 / 0	0 / 0
K	7746	4743 / 0	0 / 0	0 / 0	0 / 0	3003 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K
BEARING SIZE FACTOR = 1.15 AT JNT(S) S, K (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.03 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x6 DRY SPF No.2 T-BRACE AT D-Q, G-M
2x4 DRY SPF No.2 T-BRACE AT E-O

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 52	-78.0	-78.0	0.06 (1)	10.00	R- C	0 / 1827	0.23 (1)
B- C	-8909 / 0	-78.0	-78.0	0.91 (1)	3.88	C- Q	0 / 5433	0.48 (1)
C- D	-7810 / 0	-78.0	-78.0	0.47 (1)	3.23	Q- D	-2052 / 0	0.74 (1)
D- E	-8639 / 0	-78.0	-78.0	0.53 (1)	3.03	D- O	0 / 1863	0.16 (1)
E- F	-8639 / 0	-78.0	-78.0	0.53 (1)	3.03	O- E	-372 / 0	0.23 (1)
F- G	-8639 / 0	-78.0	-78.0	0.53 (1)	3.03	O- G	0 / 1863	0.16 (1)
G- H	-7810 / 0	-78.0	-78.0	0.47 (1)	3.23	M- G	-2052 / 0	0.74 (1)
H- I	-8909 / 0	-78.0	-78.0	0.91 (1)	3.88	M- H	0 / 5433	0.48 (1)
I- J	0 / 52	-78.0	-78.0	0.06 (1)	10.00	L- H	0 / 1827	0.23 (1)
S- B	-9426 / 0	0.0	0.0	0.47 (1)	4.91	B- R	0 / 5660	0.50 (1)
K- I	-9426 / 0	0.0	0.0	0.47 (1)	4.91	L- I	0 / 5660	0.50 (1)
S- R	0 / 0	-576.2	-576.2	0.51 (1)	10.00			
R- Q	0 / 5343	-576.2	-576.2	0.72 (1)	10.00			
Q- P	0 / 7810	-576.2	-576.2	0.60 (1)	10.00			
P- O	0 / 7810	-576.2	-576.2	0.60 (1)	10.00			
O- N	0 / 7810	-576.2	-576.2	0.60 (1)	10.00			
N- M	0 / 7810	-576.2	-576.2	0.60 (1)	10.00			
M- L	0 / 5343	-576.2	-576.2	0.72 (1)	10.00			
L- K	0 / 0	-576.2	-576.2	0.51 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 25-1-8
END DISTANCE = 32-10-0
END SPAN CARRIED = 25-1-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.09")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (1.09")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.28")

CSI: TC=0.91/1.00 (H-I:1) , BC=0.72/1.00 (L-M:1) , WB=0.74/1.00 (D-Q:1) , SSI=0.69/1.00 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.86 (P) (INPUT = 1.00)

A-18023508

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292593	H20A	1	2	TRUSS DESC.	

Alpa Roof Truss, Maple

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(table is in inches)

J.	PE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	6.0	9.0	2.25	3.00
C	TTWW+h	MT20	7.0	8.0	Edge	2.75
D	TMWW-t	MT20	4.0	4.0	2.00	1.75
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
H	TTWW+h	MT20	7.0	8.0	Edge	2.75
I	TMVW-t	MT20	6.0	9.0	2.25	3.00
K	BMV1+t	MT20	6.0	9.0	Edge	0.50
L	BMWW+t	MT20	6.0	7.0	4.25	2.00
M	BMWW+t	MT20	4.0	9.0	4.50	1.75
N	BS-t	MT20	6.0	7.0		
O	BMWWW-t	MT20	6.0	7.0	4.00	3.50
P	BS-t	MT20	5.0	6.0		
Q	BMWW+t	MT20	4.0	9.0	4.50	1.75
R	BMWW+t	MT20	6.0	7.0	4.25	2.00
S	BMV1+t	MT20	6.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

REGISTERED PROFESSIONAL ENGINEER
02/17/2018
EDWIN C. FOK
PROVINCE OF ONTARIO

A:18023508(2)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292593	H20	6	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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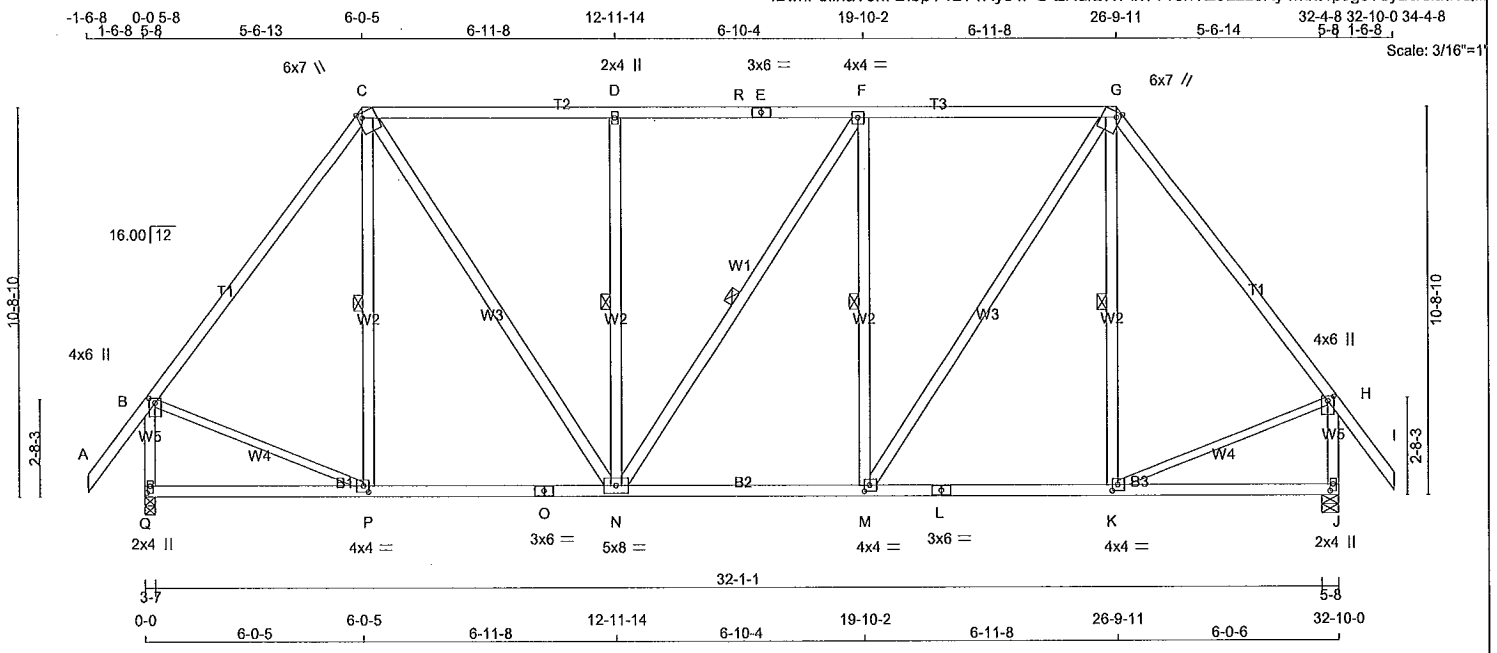


TOTAL WEIGHT = 6 X 194 = 1162 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	2100F 1.8E	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	2100F 1.8E	SPF
Q - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
WEBS	2x4	DRY	No.2	SPF
BLOCKING				
B - P	2x3	DRY	No.2	SPF
K - H	2x3	DRY	No.2	SPF

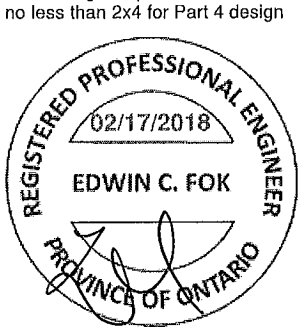
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	1.50	2.00
C	TTWW+m	MT20	6.0	7.0	Edge	1.50
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTWW+m	MT20	6.0	7.0	Edge	1.50
H	TMVW+p	MT20	4.0	6.0	1.50	2.00
J	BMV1+p	MT20	2.0	4.0	2.25	1.00
K, M, P						
K	BMWW-t	MT20	4.0	4.0	2.00	1.75
L	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	5.0	8.0		
O	BS-t	MT20	3.0	6.0		
Q	BMV1+p	MT20	2.0	4.0	2.25	1.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
Q	1791	0	1791	0	0	IN-SX 3-7	IN-SX 2-5
J	1791	0	1791	0	0	5-8	2-10

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1281	759 / 0	0 / 0	0 / 0	0 / 0	522 / 0	0 / 0
J	1281	759 / 0	0 / 0	0 / 0	0 / 0	522 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

FOR SECTION C-G, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-P, D-N, F-N, F-M, G-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 52	-78.0	-78.0	0.10 (1)	10.00	P- C	-164 / 20	0.10 (1)
B- C	-1350 / 0	-78.0	-78.0	0.28 (1)	6.25	C- N	0 / 918	0.15 (1)
C- D	-1316 / 0	-85.5	-85.5	0.52 (1)	2.00	N- D	-638 / 0	0.37 (1)
D- R	-1316 / 0	-85.5	-85.5	0.46 (1)	2.00	N- F	-1 / 0	0.00 (1)
R- E	-1316 / 0	-85.5	-85.5	0.46 (1)	2.00	M- F	-639 / 0	0.37 (1)
E- F	-1316 / 0	-85.5	-85.5	0.46 (1)	2.00	M- G	0 / 920	0.15 (1)
F- G	-1317 / 0	-85.5	-85.5	0.52 (1)	2.00	K- G	-165 / 20	0.10 (1)
G- H	-1350 / 0	-78.0	-78.0	0.28 (1)	6.25	B- P	0 / 861	0.19 (1)
H- I	0 / 52	-78.0	-78.0	0.10 (1)	10.00	K- H	0 / 861	0.19 (1)
Q- B	-1747 / 0	0.0	0.0	0.25 (1)	6.30			
J- H	-1747 / 0	0.0	0.0	0.25 (1)	6.30			
Q- P	0 / 0	-18.5	-18.5	0.19 (4)	10.00			
P- O	0 / 805	-18.5	-18.5	0.27 (4)	10.00			
O- N	0 / 805	-18.5	-18.5	0.27 (4)	10.00			
N- M	0 / 1317	-18.5	-18.5	0.30 (4)	10.00			
M- L	0 / 805	-18.5	-18.5	0.27 (4)	10.00			
L- K	0 / 805	-18.5	-18.5	0.27 (4)	10.00			
K- J	0 / 0	-18.5	-18.5	0.19 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.09")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (1.09")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.52/1.00 (F-G:1) , BC=0.30/1.00 (M-N:4) , WB=0.37/1.00 (F-M:1) , SSI=0.28/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (B) (INPUT = 0.90)
JSI METAL= 0.57 (B) (INPUT = 1.00)

A-18023507

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292593	H19	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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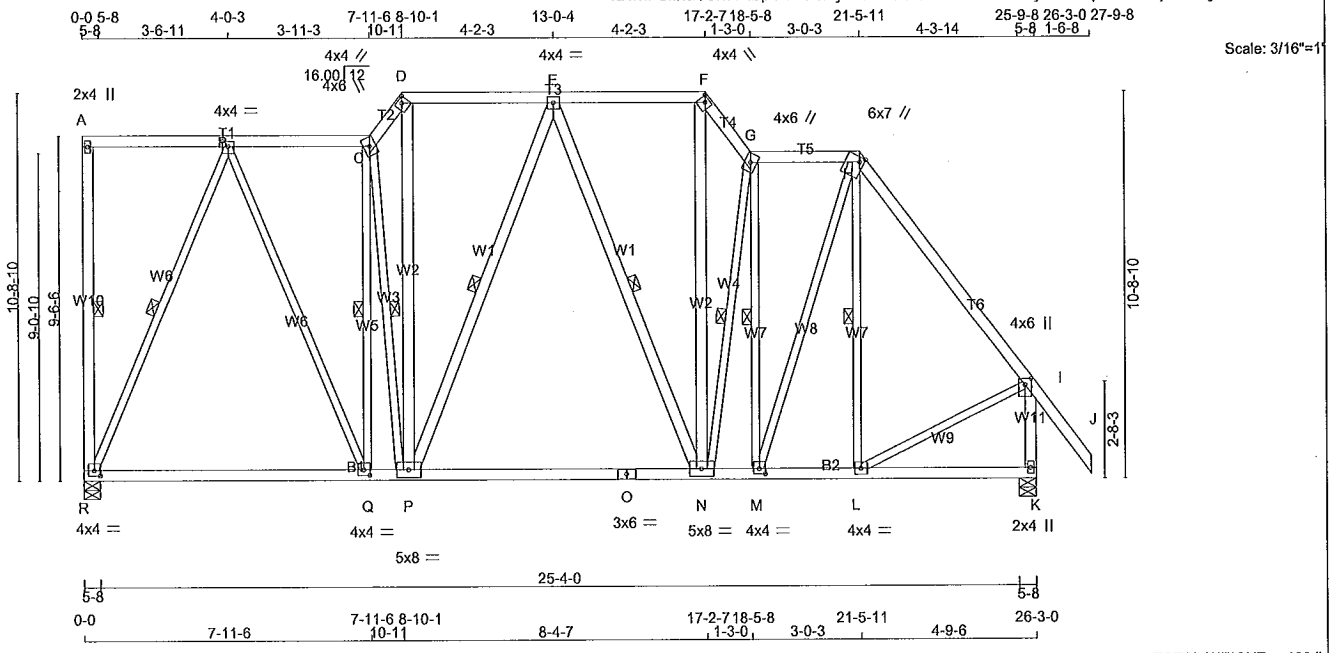


LUMBER

N. L. G. A. RULES

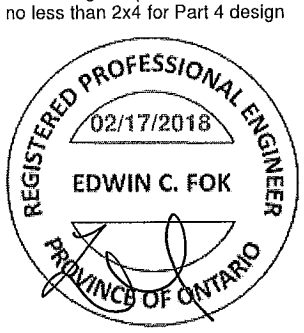
CHORDS	SIZE		LUMBER	DESCR.
R - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
WEBS EXCEPT	2x3	DRY	No.2	SPF
P - D	2x4	DRY	No.2	SPF
P - E	2x4	DRY	No.2	SPF
E - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0		
B	TMWW-t	MT20	4.0	4.0	1.75	2.00
C	TTWW+m	MT20	4.0	6.0		
D	TTW-h	MT20	4.0	4.0	Edge	
E	TMWW-t	MT20	4.0	4.0		
F	TTW-h	MT20	4.0	4.0	Edge	
G	TTWW+m	MT20	4.0	6.0		
H	TTWW+m	MT20	6.0	7.0	Edge	1.50
I	TMVW+p	MT20	4.0	6.0	2.00	2.00
K	BMV1+p	MT20	2.0	4.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMWW-t	MT20	4.0	4.0	1.75	2.00
N	BMWWW-t	MT20	5.0	8.0		
O	BS-t	MT20	3.0	6.0		
P	BMWWW-t	MT20	5.0	8.0		
Q	BMWW-t	MT20	4.0	4.0	1.75	2.00
R	BMVW1-t	MT20	4.0	4.0	1.75	2.00

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	1298	0	1298	0	0	5-8	1-8
K	1427	0	1427	0	0	5-8	2-2

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	928	551 / 0	0 / 0	0 / 0	0 / 0	377 / 0	0 / 0
K	1017	621 / 0	0 / 0	0 / 0	0 / 0	397 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

FOR SECTION D-F, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.73 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF A-R, C-Q, C-P, E-P, E-N, G-N, G-M, H-L, B-R.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
R- A	-118 / 0	0.0	0.0	0.05 (1)	6.25	Q- C	-619 / 0	0.37 (1)
A- B	0 / 0	-78.0	-78.0	0.21 (1)	10.00	C- P	-532 / 0	0.33 (1)
B- C	-795 / 0	-78.0	-78.0	0.17 (1)	6.25	P- D	0 / 809	0.13 (1)
C- D	-1237 / 0	-78.0	-78.0	0.02 (1)	5.76	P- E	-238 / 0	0.16 (1)
D- E	-753 / 0	-85.5	-85.5	0.19 (1)	2.00	E- N	-215 / 0	0.15 (1)
E- F	-762 / 0	-85.5	-85.5	0.19 (1)	2.00	N- F	0 / 806	0.13 (1)
F- G	-1252 / 0	-78.0	-78.0	0.03 (1)	5.73	N- G	-446 / 0	0.24 (1)
G- H	-813 / 0	-78.0	-78.0	0.10 (1)	6.25	M- G	-728 / 0	0.39 (1)
H- I	-994 / 0	-78.0	-78.0	0.26 (1)	5.98	M- H	0 / 698	0.16 (1)
I- J	0 / 52	-78.0	-78.0	0.15 (1)	10.00	L- H	-169 / 0	0.09 (1)
K- I	-1392 / 0	0.0	0.0	0.20 (1)	6.90	L- I	0 / 656	0.15 (1)
						R- B	-1223 / 0	0.89 (1)
R- Q	0 / 487	-18.5	-18.5	0.32 (4)	10.00	B- Q	0 / 805	0.18 (1)
Q- P	0 / 802	-18.5	-18.5	0.35 (4)	10.00			
P- O	0 / 840	-18.5	-18.5	0.29 (4)	10.00			
O- N	0 / 840	-18.5	-18.5	0.29 (4)	10.00			
N- M	0 / 822	-18.5	-18.5	0.29 (4)	10.00			
M- L	0 / 592	-18.5	-18.5	0.17 (4)	10.00			
L- K	0 / 0	-18.5	-18.5	0.11 (4)	10.00			

[M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN HIGHEST FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

LOADING IN OTHER FLAT SECTIONS BASED ON A SLOPE OF 6.00/12 AND 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.88")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.88")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.26/1.00 (H-I:1) , BC=0.35/1.00 (P-Q:4) , WB=0.89/1.00 (B-R:1) , SSI=0.17/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (M) (INPUT = 0.90)
JSI METAL= 0.41 (D) (INPUT = 1.00)

A-18023506

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292593	H18	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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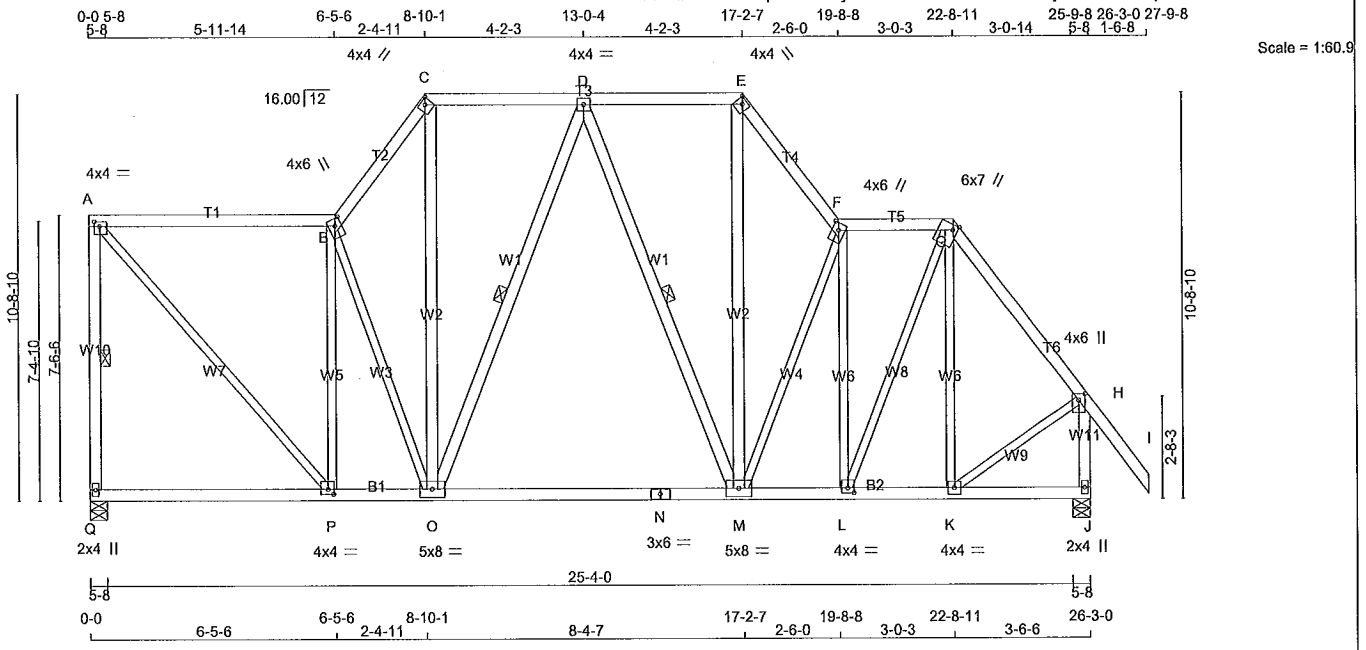


TOTAL WEIGHT = 174 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
Q - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
J	2x4	DRY	No.2	SPF
WEBS	2x3	DRY	No.2	SPF
EXCEPT				
O - C	2x4	DRY	No.2	SPF
O - D	2x4	DRY	No.2	SPF
D - M	2x4	DRY	No.2	SPF
M - E	2x4	DRY	No.2	SPF

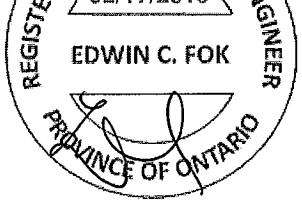
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	4.0	1.50	1.75
B	TTWW+m	MT20	4.0	6.0	2.25	2.00
C	TTW-h	MT20	4.0	4.0	Edge	
D	TMWW-t	MT20	4.0	4.0		
E	TTW-h	MT20	4.0	4.0	Edge	
F	TTWW+m	MT20	4.0	6.0	2.25	2.00
G	TTWW+m	MT20	6.0	7.0	Edge	1.50
H	TMVW+p	MT20	4.0	6.0	2.00	2.00
J	BMV1+p	MT20	2.0	4.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMWW-t	MT20	4.0	4.0	1.50	2.00
M	BMWWW-t	MT20	5.0	8.0		
N	BS-t	MT20	3.0	6.0		
O	BMWWW-t	MT20	5.0	8.0		
P	BMWW-t	MT20	4.0	4.0	1.50	1.75
Q	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
Q	1298	0	1298	0	0	5-8	1-15
J	1427	0	1427	0	0	5-8	2-2

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	928	551 / 0	0 / 0	0 / 0	0 / 0	377 / 0	0 / 0
J	1017	621 / 0	0 / 0	0 / 0	0 / 0	397 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

FOR SECTION C-E, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.68 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF A-Q, D-O, D-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- A	-1247 / 0	0.0	0.0	0.32 (1)	5.76	A- P	0 / 1332	0.30 (1)
A- B	-878 / 0	-78.0	-78.0	0.43 (1)	5.95	P- B	-920 / 0	0.95 (1)
B- C	-1237 / 0	-78.0	-78.0	0.08 (1)	5.71	B- O	-447 / 0	0.51 (1)
C- D	-752 / 0	-85.5	-85.5	0.19 (1)	2.00	O- C	0 / 750	0.12 (1)
D- E	-762 / 0	-85.5	-85.5	0.19 (1)	2.00	O- D	-240 / 0	0.17 (1)
E- F	-1253 / 0	-78.0	-78.0	0.08 (1)	5.68	D- M	-213 / 0	0.15 (1)
F- G	-896 / 0	-78.0	-78.0	0.10 (1)	6.25	M- E	0 / 759	0.12 (1)
G- H	-964 / 0	-78.0	-78.0	0.14 (1)	6.19	M- F	-444 / 0	0.49 (1)
H- I	0 / 52	-78.0	-78.0	0.15 (1)	10.00	L- F	-802 / 0	0.78 (1)
J- H	-1403 / 0	0.0	0.0	0.20 (1)	6.87	L- G	0 / 852	0.19 (1)
						K- G	-269 / 0	0.26 (1)
Q- P	0 / 0	-18.5	-18.5	0.16 (4)	10.00	K- H	0 / 679	0.15 (1)
P- O	0 / 891	-18.5	-18.5	0.27 (4)	10.00			
O- N	0 / 839	-18.5	-18.5	0.29 (4)	10.00			
N- M	0 / 839	-18.5	-18.5	0.29 (4)	10.00			
M- L	0 / 908	-18.5	-18.5	0.30 (4)	10.00			
L- K	0 / 570	-18.5	-18.5	0.14 (1)	10.00			
K- J	0 / 0	-18.5	-18.5	0.07 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN HIGHEST FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

LOADING IN OTHER FLAT SECTIONS BASED ON A SLOPE OF 6.00/12 AND 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.88")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.88")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.43/1.00 (A-B:1) , BC=0.30/1.00 (L-M:4) , WB=0.95/1.00 (B-P:1) , SSI=0.20/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (A) (INPUT = 0.90)
JSI METAL= 0.39 (E) (INPUT = 1.00)

A-18023505

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292593	H17	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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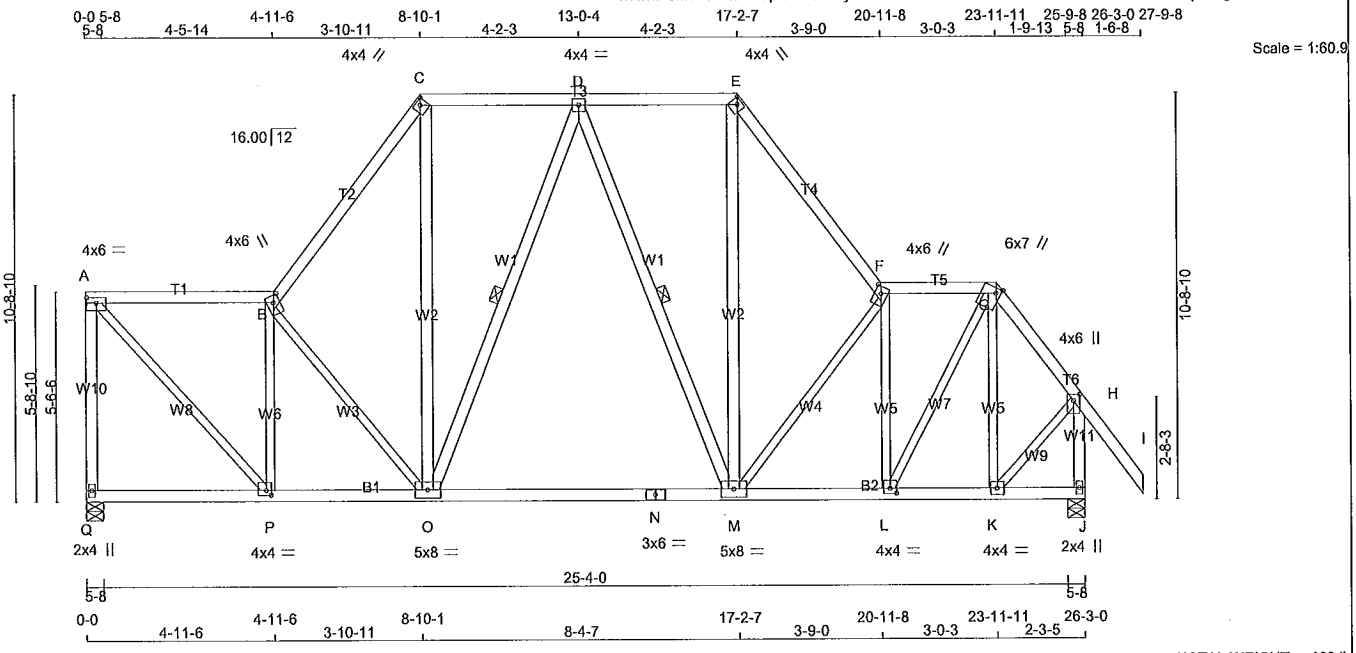


TOTAL WEIGHT = 163 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
Q - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
J	2x4	DRY	No.2	SPF
WEBS	2x3	DRY	No.2	SPF
EXCEPT				
O - C	2x4	DRY	No.2	SPF
O - D	2x4	DRY	No.2	SPF
D - M	2x4	DRY	No.2	SPF
M - E	2x4	DRY	No.2	SPF

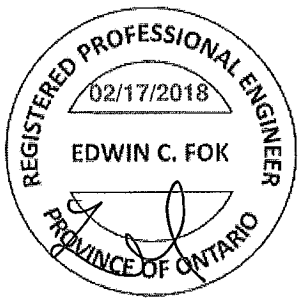
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0	1.75	3.00
B	TTWW+m	MT20	4.0	6.0	2.25	2.00
C	TTW-h	MT20	4.0	4.0	Edge	
D	TMWW-t	MT20	4.0	4.0		
E	TTW-h	MT20	4.0	4.0	Edge	
F	TTWW+m	MT20	4.0	6.0	2.25	2.00
G	TTWW+m	MT20	6.0	7.0	Edge	1.50
H	TMVW+p	MT20	4.0	6.0	2.00	2.00
J	BMV1+p	MT20	2.0	4.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMWW-t	MT20	4.0	4.0	1.50	2.00
M	BMWWW-t	MT20	5.0	8.0		
N	BS-t	MT20	3.0	6.0		
O	BMWWW-t	MT20	5.0	8.0		
P	BMWW-t	MT20	4.0	4.0	1.50	1.50
Q	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
Q	1294	0	1294	0	0	IN-SX	IN-SX
J	1431	0	1431	0	0	5-8	1-15
						5-8	2-2

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	925	549 / 0	0 / 0	0 / 0	0 / 0	376 / 0	0 / 0
J	1020	623 / 0	0 / 0	0 / 0	0 / 0	397 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

FOR SECTION C-E, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.60 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-O, D-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
Q- A	-1252 / 0	0.0	0.0	0.69 (1)	7.19
A- B	-987 / 0	-78.0	-78.0	0.26 (1)	5.97
B- C	-1234 / 0	-78.0	-78.0	0.18 (1)	5.60
C- D	-749 / 0	-85.5	-85.5	0.19 (1)	2.00
D- E	-755 / 0	-85.5	-85.5	0.19 (1)	2.00
E- F	-1243 / 0	-78.0	-78.0	0.17 (1)	5.60
F- G	-988 / 0	-78.0	-78.0	0.10 (1)	6.17
G- H	-878 / 0	-78.0	-78.0	0.12 (1)	6.25
H- I	0 / 52	-78.0	-78.0	0.15 (1)	10.00
J- H	-1421 / 0	0.0	0.0	0.21 (1)	6.84
Q- P	0 / 0	-18.5	-18.5	0.10 (4)	10.00
P- O	0 / 1007	-18.5	-18.5	0.29 (4)	10.00
O- N	0 / 835	-18.5	-18.5	0.28 (4)	10.00
N- M	0 / 835	-18.5	-18.5	0.28 (4)	10.00
M- L	0 / 1006	-18.5	-18.5	0.30 (4)	10.00
L- K	0 / 491	-18.5	-18.5	0.12 (1)	10.00
K- J	0 / 0	-18.5	-18.5	0.05 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- P	0 / 1455	0.33 (1)
P- B	-998 / 0	0.46 (1)
B- O	-435 / 0	0.33 (1)
O- C	0 / 689	0.11 (1)
O- D	-235 / 0	0.16 (1)
D- M	-219 / 0	0.15 (1)
M- E	0 / 702	0.11 (1)
M- F	-443 / 0	0.35 (1)
L- F	-899 / 0	0.45 (1)
L- G	0 / 1045	0.24 (1)
K- G	-403 / 0	0.20 (1)
K- H	0 / 692	0.16 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN HIGHEST FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

LOADING IN OTHER FLAT SECTIONS BASED ON A SLOPE OF 6.00/12 AND 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.87")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.87")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.69/1.00 (A-Q:1) , BC=0.30/1.00 (L-M:4) , WB=0.46/1.00 (B-P:1) , SSI=0.17/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (A) (INPUT = 0.90)
JSI METAL= 0.38 (P) (INPUT = 1.00)

A-18023504

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292593	H16	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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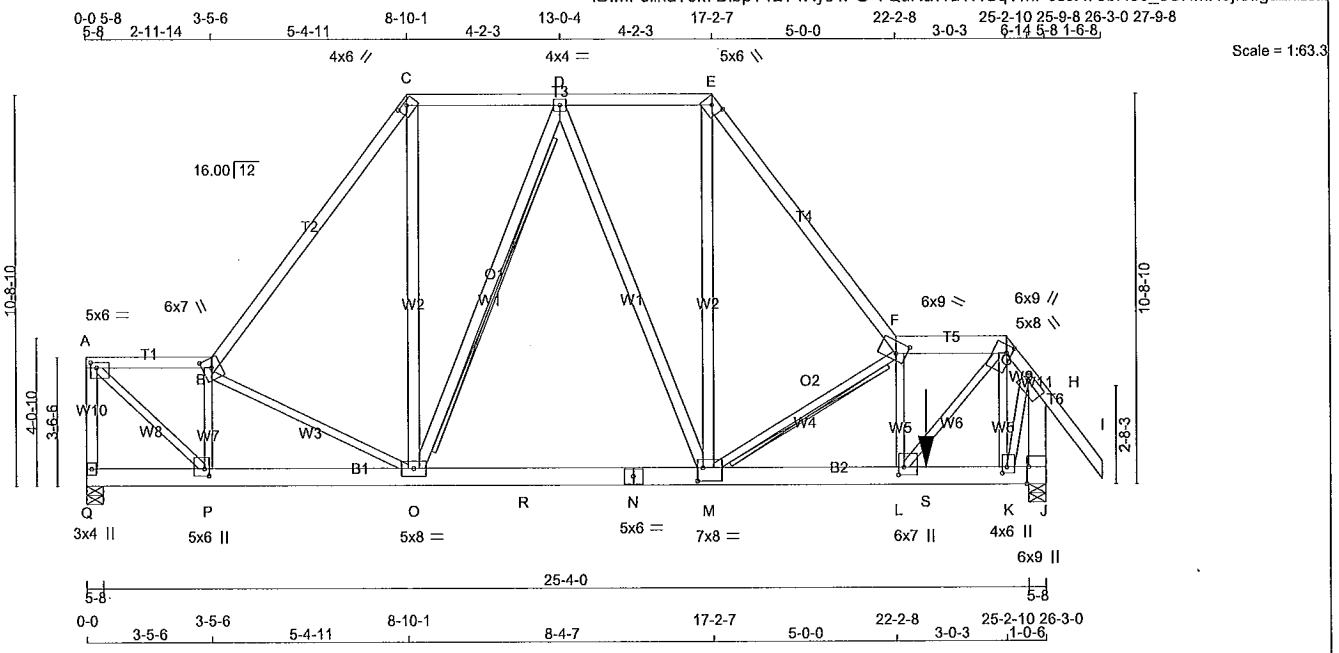


TOTAL WEIGHT = 172 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
Q - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	1650F 1.5E	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	1650F 1.5E	SPF
F - G	2x6	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
Q - N	2x6	DRY	1650F 1.5E	SPF
N - I	2x6	DRY	1650F 1.5E	SPF
WEBS	2x3	DRY	No.2	SPF
EXCEPT				
O - C	2x4	DRY	No.2	SPF
O - D	2x4	DRY	No.2	SPF
D - M	2x4	DRY	No.2	SPF
M - E	2x4	DRY	No.2	SPF

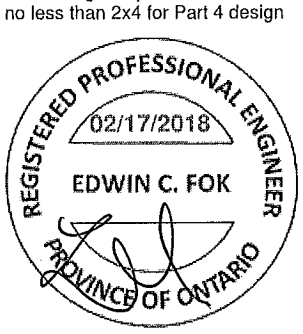
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0	1.75	2.00
B	TTWW+m	MT20	6.0	7.0	3.25	3.00
C	TTW-h	MT20	4.0	6.0	1.25	3.00
D	TMWW-t	MT20	4.0	4.0		
E	TTW-h	MT20	5.0	6.0	2.00	3.25
F	TTWW-m	MT20	6.0	9.0	3.75	3.25
G	TTWW+m	MT20	6.0	9.0	2.50	1.75
H	TMVW-t	MT20	5.0	8.0	2.00	2.00
J	BMV1+t	MT20	6.0	9.0	Edge	0.50
K	BMWW+t	MT20	4.0	6.0	2.00	1.50
L	BMWW+t	MT20	6.0	7.0	2.50	1.75
M	BMWWW-t	MT20	7.0	8.0	4.25	1.75
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	8.0		
P	BMWW+t	MT20	5.0	6.0	2.25	1.50
Q	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	2191	0	2191	0	0	5-8	3-10
J	3672	0	3672	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1565	939 / 0	0 / 0	0 / 0	0 / 0	626 / 0	0 / 0
J	2618	1598 / 0	0 / 0	0 / 0	0 / 0	1020 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BEARING SIZE FACTOR = 1.15 AT JNT(S) J (BASED ON SUPPORT DEPTH = 1-8)

BRACING

FOR SECTION C-E, MAX. PURLIN SPACING = 2.00 FT.

FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.91 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT D-O, F-M

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
Q- A	-2082 / 0	0.0	0.0	0.39 (1)	5.81
A- B	-2092 / 0	-78.0	-78.0	0.24 (1)	4.43
B- C	-2594 / 0	-78.0	-78.0	0.67 (1)	4.33
C- D	-1580 / 0	-85.5	-85.5	0.28 (1)	2.00
D- E	-2060 / 0	-85.5	-85.5	0.31 (1)	2.00
E- F	-3392 / 0	-78.0	-78.0	0.68 (1)	3.91
F- G	-3510 / 0	-78.0	-78.0	0.13 (1)	4.44
G- H	-1766 / 0	-78.0	-78.0	0.20 (1)	4.79
H- I	0 / 52	-78.0	-78.0	0.17 (1)	10.00
J- H	-3943 / 0	0.0	0.0	0.40 (1)	5.36
Q- P	0 / 0	-18.5	-18.5	0.13 (1)	10.00
P- O	0 / 2163	-18.5	-18.5	0.44 (1)	10.00
O- R	0 / 1897	-18.5	-18.5	0.66 (1)	10.00
R- N	0 / 1897	-239.3	-239.3	0.66 (1)	10.00
N- M	0 / 1897	-239.3	-239.3	0.66 (1)	10.00
M- L	0 / 3567	-239.3	-239.3	0.79 (1)	10.00
L- S	0 / 1040	-239.3	-239.3	0.25 (1)	10.00
S- K	0 / 1040	-18.5	-18.5	0.25 (1)	10.00
K- J	0 / 0	-18.5	-18.5	0.14 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- P	0 / 2879	0.71 (1)
P- B	-2110 / 0	0.44 (1)
B- O	-695 / 0	0.50 (1)
O- C	0 / 1705	0.30 (1)
O- D	-869 / 0	0.78 (1)
D- M	0 / 450	0.08 (1)
M- E	0 / 2359	0.42 (1)
M- F	-1878 / 0	0.58 (1)
L- F	-1958 / 0	0.46 (1)
L- G	0 / 3920	0.97 (1)
K- G	-1750 / 0	0.41 (1)
K- H	0 / 2529	0.63 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
S	23-0-4	-704	-704	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN HIGHEST FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

LOADING IN OTHER FLAT SECTIONS BASED ON A SLOPE OF 6.00/12 AND 6.00/12

GIRDER TYPE: CStdGirder
START DISTANCE = 12-0-0
START SPAN CARRIED = 11-1-14
END DISTANCE = 23-0-4
END SPAN CARRIED = 11-1-14
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.87")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (0.87")
CALCULATED VERT. DEFL.(TL) = L/ 859 (0.37")

CSI: TC=0.68/1.00 (E-F:1) , BC=0.79/1.00 (L-M:1) , WB=0.97/1.00 (G-L:1) , SSI=0.60/1.00 (M-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

A-18023503

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292593	H15P	4	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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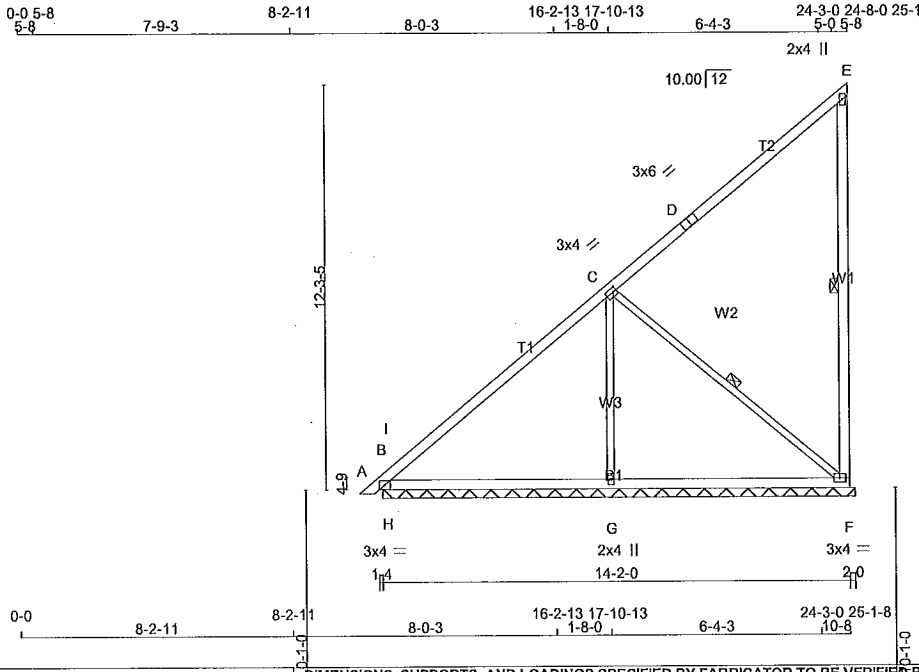


TOTAL WEIGHT = 4 X 63 = 254 lb

[M]

LUMBER

N. L. G. A. RULES

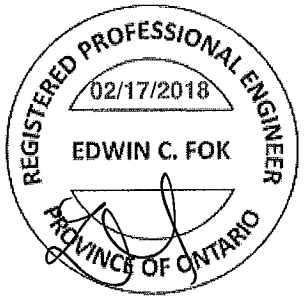
CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

.S (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TMWW-t	MT20	3.0	4.0	1.50	1.25
D	TS-t	MT20	3.0	6.0		
E	TMV+p	MT20	2.0	4.0		
F	BMVW1-t	MT20	3.0	4.0		
G	BMW1+w	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	396	0	396	0	0	14-4-0 (14-2-0)	1-15
B	429	0	429	0	0	14-4-0 (14-2-0)	1-15
G	592	0	592	0	0	14-4-0 (14-2-0)	1-15

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	280	183 / 0	0 / 0	0 / 0	0 / 0	97 / 0	0 / 0
B	303	202 / 0	0 / 0	0 / 0	0 / 0	101 / 0	0 / 0
G	426	236 / 0	0 / 0	0 / 0	0 / 0	190 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, C-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 12	-78.0	-78.0	0.02 (1)	10.00	G- C	-399 / 0	0.25 (1)
B- I	-81 / 0	-78.0	-78.0	0.11 (1)	6.25	C- F	-209 / 0	0.12 (1)
I- C	-174 / 0	-78.0	-78.0	0.65 (1)	6.25	H- I	-500 / 0	0.00 (1)
C- D	-41 / 0	-78.0	-78.0	0.63 (1)	6.25			
D- E	-41 / 0	-78.0	-78.0	0.63 (1)	6.25			
F- E	-217 / 0	0.0	0.0	0.18 (1)	6.25			
B- H	0 / 162	-18.5	-18.5	0.21 (1)	10.00			
H- G	0 / 162	-18.5	-18.5	0.29 (4)	10.00			
G- F	0 / 162	-18.5	-18.5	0.29 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.65/1.00 (C-I:1) , BC=0.29/1.00 (G-H:4) , WB=0.25/1.00 (C-G:1) , SSI=0.41/1.00 (B-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (D) (INPUT = 0.90)
JSI METAL= 0.36 (D) (INPUT = 1.00)

A-18023502

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292593	H14P	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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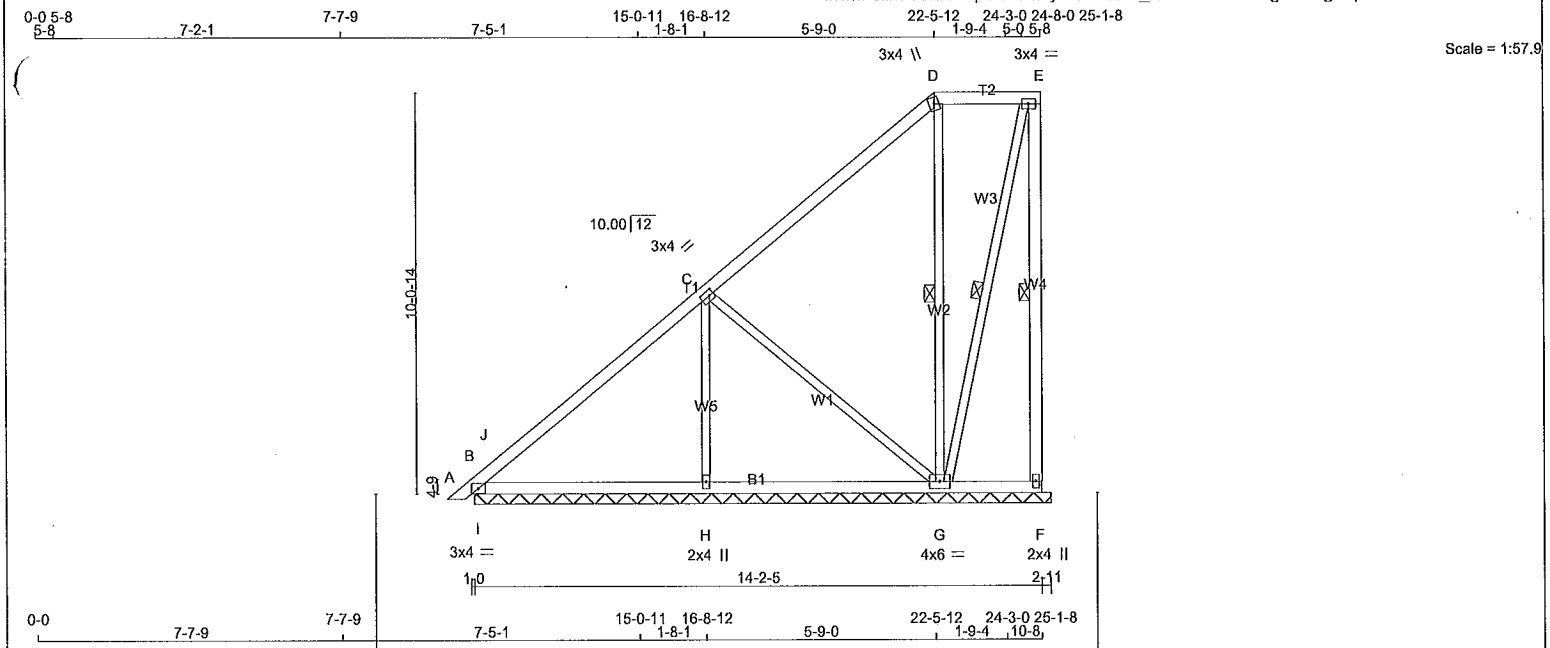


TOTAL WEIGHT = 74 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

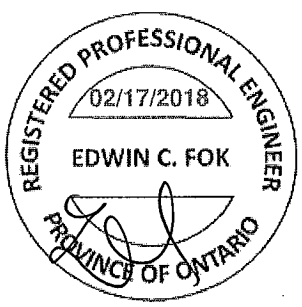
DRY: SEASONED LUMBER.

10-1-0

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-l	MT20	3.0	4.0		
C	TMWW-t	MT20	3.0	4.0	1.50	1.25
D	TTW+m	MT20	3.0	4.0		
E	TMVW-t	MT20	3.0	4.0		
F	BMV1+p	MT20	2.0	4.0		
G	BMWWW1-t	MT20	4.0	6.0		
H	BMW1+w	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	92	0	92	0	0	14-5-0 (14-2-5)	6-9
B	362	0	362	0	0	14-5-0 (14-2-5)	6-9
G	488	0	488	0	0	14-5-0 (14-2-5)	6-9
H	476	0	476	0	0	14-5-0 (14-2-5)	6-9

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	64	44 / 0	0 / 0	0 / 0	0 / 0	21 / 0	0 / 0
B	256	169 / 0	0 / 0	0 / 0	0 / 0	86 / 0	0 / 0
G	347	214 / 0	0 / 0	0 / 0	0 / 0	133 / 0	0 / 0
H	342	194 / 0	0 / 0	0 / 0	0 / 0	148 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, G, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, D-G, E-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 12	-78.0	-78.0	0.02 (1)	10.00
B- J	-71 / 0	-78.0	-78.0	0.07 (1)	6.25
J- C	-136 / 0	-78.0	-78.0	0.42 (1)	6.25
C- D	-29 / 0	-78.0	-78.0	0.42 (1)	6.25
D- E	0 / 6	-78.0	-78.0	0.09 (1)	10.00
F- E	-78 / 0	0.0	0.0	0.04 (1)	6.25
B- I	0 / 127	-18.5	-18.5	0.16 (1)	10.00
I- H	0 / 127	-18.5	-18.5	0.17 (4)	10.00
H- G	0 / 127	-18.5	-18.5	0.17 (4)	10.00
G- F	0 / 0	-18.5	-18.5	0.08 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- D	-277 / 0	0.19 (1)
H- C	-322 / 0	0.13 (1)
C- G	-170 / 0	0.20 (1)
G- E	-26 / 0	0.02 (1)
I- J	-364 / 6	0.00 (1)

10-1-0

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	21.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.42/1.00 (C-J:1) , BC=0.17/1.00 (H-I:4) , WB=0.20/1.00 (C-G:1) , SSI=0.30/1.00 (B-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.38 (C) (INPUT = 0.90)
JSI METAL= 0.10 (D) (INPUT = 1.00)

A-18023501

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292593	H13P	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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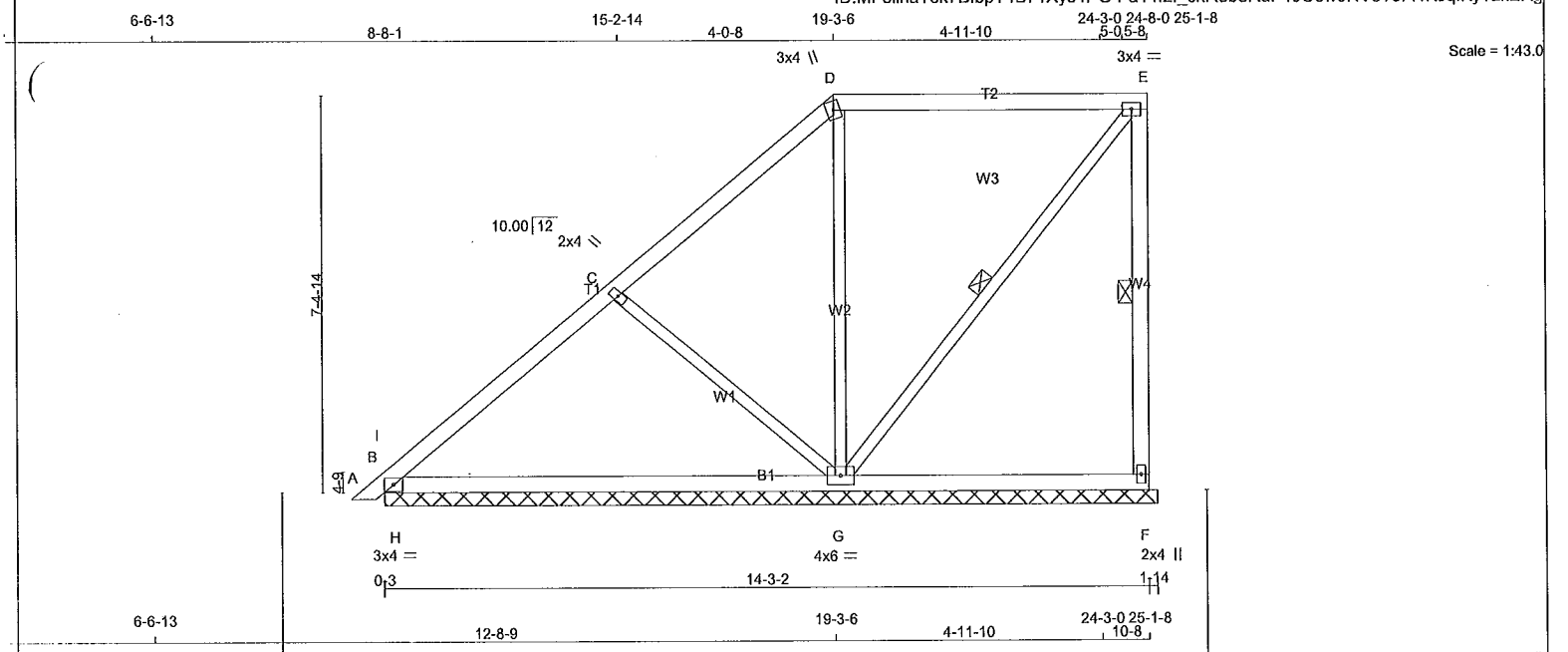


TOTAL WEIGHT = 61 lb

[M]

LUMBER

N. L. G. A. RULES

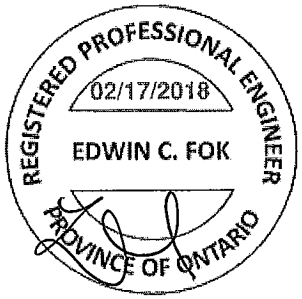
CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

.3 (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TMW+w	MT20	2.0	4.0		
D	TTW+m	MT20	3.0	4.0		
E	TMVW-t	MT20	3.0	4.0		
F	BMV1+p	MT20	2.0	4.0		
G	BMWWW1-t	MT20	4.0	6.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	222	0	222	0	0	14-5-0 (14-3-2)	8-13
B	415	0	415	0	0	14-5-0 (14-3-2)	8-13
G	780	0	780	0	0	14-5-0 (14-3-2)	8-13

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	158	98 / 0	0 / 0	0 / 0	0 / 0	60 / 0	0 / 0
B	295	186 / 0	0 / 0	0 / 0	0 / 0	109 / 0	0 / 0
G	557	336 / 0	0 / 0	0 / 0	0 / 0	220 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, E-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 12	-78.0	-78.0	0.02 (1)	10.00	G- D	-367 / 0	0.36 (1)
B- I	-458 / 0	-78.0	-78.0	0.16 (4)	6.25	C- G	-326 / 0	0.17 (1)
I- C	-253 / 0	-78.0	-78.0	0.25 (1)	6.25	G- E	-48 / 0	0.03 (1)
C- D	-6 / 5	-78.0	-78.0	0.24 (1)	10.00	H- I	0 / 300	0.00 (1)
D- E	0 / 30	-78.0	-78.0	0.46 (1)	10.00			
F- E	-190 / 0	0.0	0.0	0.05 (1)	6.25			
B- H	0 / 223	-18.5	-18.5	0.14 (4)	10.00			
H- G	0 / 223	-18.5	-18.5	0.28 (4)	10.00			
G- F	0 / 0	-18.5	-18.5	0.26 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.46/1.00 (D-E:1) , BC=0.28/1.00 (G-H:4) , WB=0.36/1.00 (D-G:1) , SSI=0.27/1.00 (B-H:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.47 (D) (INPUT = 0.90)
JSI METAL= 0.11 (D) (INPUT = 1.00)

A-18023500

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292593	H12P	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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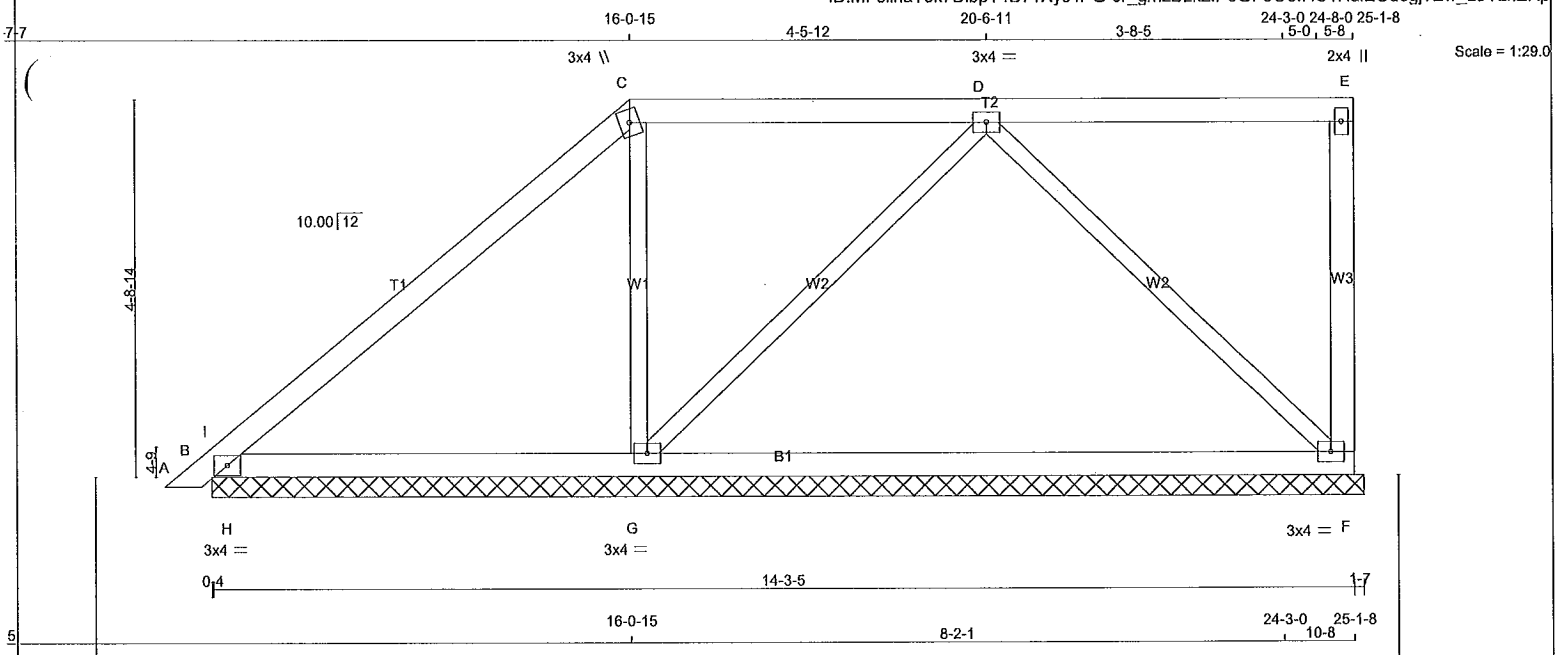


TOTAL WEIGHT = 53 lb

LUMBER
N. L. G. A RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW-m	MT20	3.0	4.0		
D	TMWW-t	MT20	3.0	4.0		
E	TMV-p	MT20	2.0	4.0		
F	BMVW1-t	MT20	3.0	4.0		
G	BMWW1-t	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	439	0	439	0	0	14-5-0 (14-3-02)1-2	
B	310	0	310	0	0	14-5-0 (14-3-02)1-2	
G	669	0	669	0	0	14-5-0 (14-3-02)1-2	

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	311	200 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0
B	217	155 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0
G	482	266 / 0	0 / 0	0 / 0	0 / 0	216 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, G.

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 12	-78.0	-78.0	0.02 (1)	10.00	G- C	-254 / 0	0.08 (1)
B- I	0 / 229	-78.0	-78.0	0.19 (1)	10.00	G- D	-279 / 0	0.20 (1)
I- C	-89 / 0	-78.0	-78.0	0.26 (1)	6.25	D- F	-344 / 0	0.24 (1)
C- D	-51 / 0	-78.0	-78.0	0.27 (1)	6.25	H- I	-655 / 0	0.00 (1)
D- E	0 / 0	-78.0	-78.0	0.27 (1)	10.00			
F- E	-135 / 0	0.0	0.0	0.05 (1)	7.81			
B- H	0 / 57	-18.5	-18.5	0.24 (1)	10.00			
H- G	0 / 57	-18.5	-18.5	0.35 (4)	10.00			
G- F	0 / 246	-18.5	-18.5	0.36 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.27/1.00 (C-D:1) , BC=0.36/1.00 (F-G:4) , WB=0.24/1.00 (D-F:1) , SSI=0.50/1.00 (B-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.44 (F) (INPUT = 0.90)
JSI METAL= 0.09 (F) (INPUT = 1.00)

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

REGISTERED PROFESSIONAL ENGINEER
02/17/2018
EDWIN C. FOK
PROVINCE OF ONTARIO

A-18023499

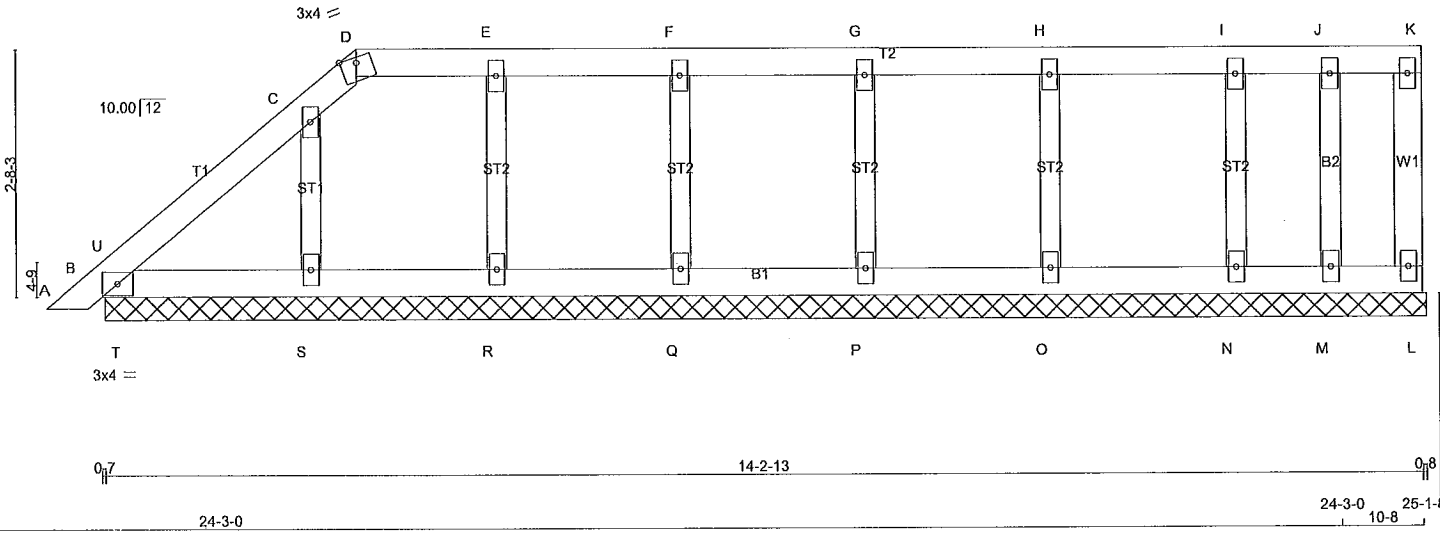
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292593	H11P	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

ID:MF8ilriaTok7DlbpT4B71Xys4PG-BhdPVw6a87YGlLo?1NYtym1wepPOE?a6s_X1wrzkERv

13-7-6 | 10-7-10 | 24-3-0 | 24-8-0 | 25-1-8 | 5-0 | 5-8

Scale = 1:25.0



TOTAL WEIGHT = 48 lb

[M]

10-1-0

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - K	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
M - J	2x3	DRY	No.2	SPF
B - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

SEASONED LUMBER.

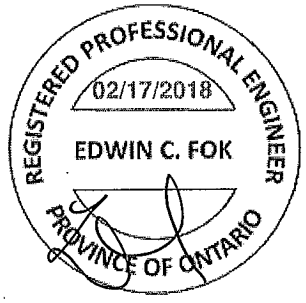
E STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C, E, F, G, H, I						
C	TMW+w	MT20	2.0	4.0		
D	TT-m	MT20	3.0	4.0	Edge	
J	TMV+p	MT20	2.0	4.0		
K	TMV+p	MT20	2.0	4.0		
L	BMV1+p	MT20	2.0	4.0		
M	BMV1+p	MT20	2.0	4.0		
N, O, P, Q, R, S						
N	BMW1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 12	-78.0	-78.0	0.02 (1)	10.00	N- I	-126 / 0	0.02 (1)
B- U	-12 / 0	-78.0	-78.0	0.01 (4)	6.25	O- H	-161 / 0	0.03 (1)
U- C	0 / 1	-78.0	-78.0	0.05 (1)	10.00	P- G	-155 / 0	0.03 (1)
C- D	-42 / 0	-78.0	-78.0	0.05 (1)	6.25	Q- F	-155 / 0	0.03 (1)
D- E	0 / 0	-78.0	-78.0	0.04 (1)	10.00	R- E	-156 / 0	0.03 (1)
E- F	0 / 0	-78.0	-78.0	0.04 (1)	10.00	S- C	-173 / 0	0.03 (1)
F- G	0 / 0	-78.0	-78.0	0.04 (1)	10.00	T- U	-65 / 0	0.00 (1)
G- H	0 / 0	-78.0	-78.0	0.04 (1)	10.00			
H- I	0 / 0	-78.0	-78.0	0.04 (1)	10.00			
I- J	0 / 0	-78.0	-78.0	0.02 (1)	10.00			
J- K	0 / 0	-78.0	-78.0	0.01 (1)	10.00			
L- K	-33 / 0	0.0	0.0	0.00 (1)	7.81			
M- J	-71 / 0	0.0	0.0	0.01 (1)	7.81			
B- T	0 / 9	-18.5	-18.5	0.03 (1)	10.00			
T- S	0 / 9	-18.5	-18.5	0.03 (1)	10.00			
S- R	0 / 0	-18.5	-18.5	0.02 (4)	10.00			
R- Q	0 / 0	-18.5	-18.5	0.02 (4)	10.00			
Q- P	0 / 0	-18.5	-18.5	0.02 (4)	10.00			
P- O	0 / 0	-18.5	-18.5	0.02 (4)	10.00			
O- N	0 / 0	-18.5	-18.5	0.02 (4)	10.00			
N- M	0 / 0	-18.5	-18.5	0.01 (4)	10.00			
M- L	0 / 0	-18.5	-18.5	0.00 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.05/1.00 (C-U:1) , BC=0.03/1.00 (S-T:1) , WB=0.03/1.00 (H-O:1) , SSI=0.06/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (C) (INPUT = 0.90)
JSI METAL= 0.09 (C) (INPUT = 1.00)

A-18023498

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292593	H10G	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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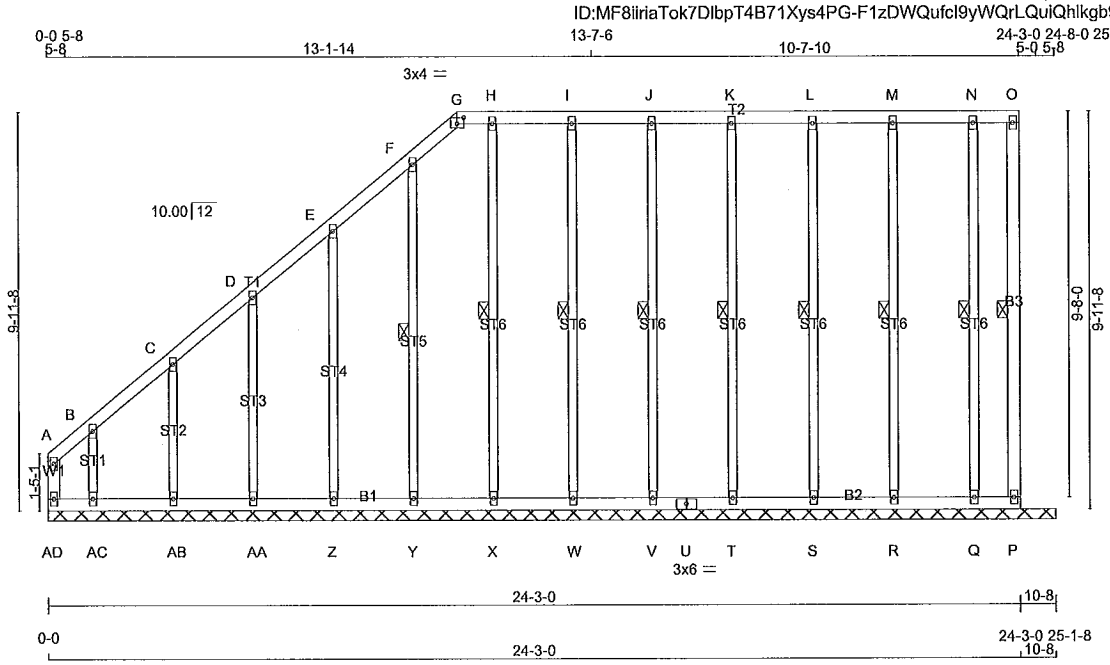


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
AD- A	2x4	DRY	No.2	SPF
A - G	2x4	DRY	No.2	SPF
G - O	2x4	DRY	No.2	SPF
AD- U	2x4	DRY	No.2	SPF
U - P	2x4	DRY	No.2	SPF
P - O	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
\BLE WEBS	2x3	DRY	No.2	SPF

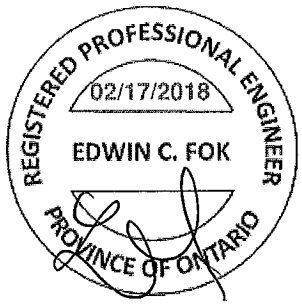
DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0		
B, C, D, E, F, H, I, J, K, L, M, N						
B	TMW+w	MT20	2.0	4.0		
G	TT-I	MT20	3.0	4.0	1.75	2.00
O	TMV+p	MT20	2.0	4.0		
P	BMV1+p	MT20	2.0	4.0		
Q, R, S, T, V, W, X, Y, Z, AA, AB, AC						
Q	BMW1+w	MT20	2.0	4.0		
U	BS-t	MT20	3.0	6.0		
AD	BMV1+p	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

FOR SECTION G-O, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF O-P, N-Q, M-R, L-S, K-T, J-V, I-W, H-X, F-Y.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
AD- A	-46 / 0	0.0	0.0	0.01 (1)		7.81	Q- N	-128 / 0	0.09 (1)
A- B	-17 / 0	-78.0	-78.0	0.02 (1)		6.25	R- M	-180 / 0	0.12 (1)
B- C	-12 / 0	-78.0	-78.0	0.04 (1)		6.25	S- L	-170 / 0	0.11 (1)
C- D	-10 / 0	-78.0	-78.0	0.04 (1)		6.25	T- K	-171 / 0	0.11 (1)
D- E	-6 / 0	-78.0	-78.0	0.04 (1)		10.00	V- J	-171 / 0	0.11 (1)
E- F	-3 / 0	-78.0	-78.0	0.04 (1)		10.00	W- I	-170 / 0	0.11 (1)
F- G	-23 / 0	-78.0	-78.0	0.04 (1)		6.25	X- H	-173 / 0	0.12 (1)
G- H	0 / 0	-85.5	-85.5	0.04 (1)		2.00	Y- F	-159 / 0	0.08 (1)
H- I	0 / 0	-85.5	-85.5	0.04 (1)		2.00	Z- E	-154 / 0	0.15 (1)
I- J	0 / 0	-85.5	-85.5	0.04 (1)		2.00	AA- D	-153 / 0	0.08 (1)
J- K	0 / 0	-85.5	-85.5	0.04 (1)		2.00	AB- C	-158 / 0	0.04 (1)
K- L	0 / 0	-85.5	-85.5	0.04 (1)		2.00	AC- B	-117 / 0	0.02 (1)
L- M	0 / 0	-85.5	-85.5	0.04 (1)		2.00			
M- N	0 / 0	-85.5	-85.5	0.04 (1)		2.00			
N- O	0 / 0	-85.5	-85.5	0.02 (1)		2.00			
AD-AC	0 / 17	-18.5	-18.5	0.01 (1)		10.00			
AC-AB	0 / 11	-18.5	-18.5	0.02 (4)		10.00			
AB-AA	0 / 7	-18.5	-18.5	0.02 (4)		10.00			
AA- Z	0 / 5	-18.5	-18.5	0.01 (4)		10.00			
Z- Y	0 / 2	-18.5	-18.5	0.01 (4)		10.00			
Y- X	0 / 0	-18.5	-18.5	0.01 (4)		10.00			
X- W	0 / 0	-18.5	-18.5	0.01 (4)		10.00			
W- V	0 / 0	-18.5	-18.5	0.01 (4)		10.00			
V- U	0 / 0	-18.5	-18.5	0.01 (4)		10.00			
U- T	0 / 0	-18.5	-18.5	0.01 (4)		10.00			
T- S	0 / 0	-18.5	-18.5	0.01 (4)		10.00			
S- R	0 / 0	-18.5	-18.5	0.02 (4)		10.00			
R- Q	0 / 0	-18.5	-18.5	0.02 (4)		10.00			
Q- P	0 / 0	-18.5	-18.5	0.01 (4)		10.00			
P- O	-33 / 0	0.0	0.0	0.01 (1)		6.25			

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.04/1.00 (L-M:1) , BC=0.02/1.00 (AB-AC:4) , WB=0.15/1.00 (E-Z:1) , SSI=0.07/1.00 (M-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)		SHEAR (PSI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.64 (G) (INPUT = 0.90)
JSI METAL= 0.08 (F) (INPUT = 1.00)

A-18023497

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292593	H10	7	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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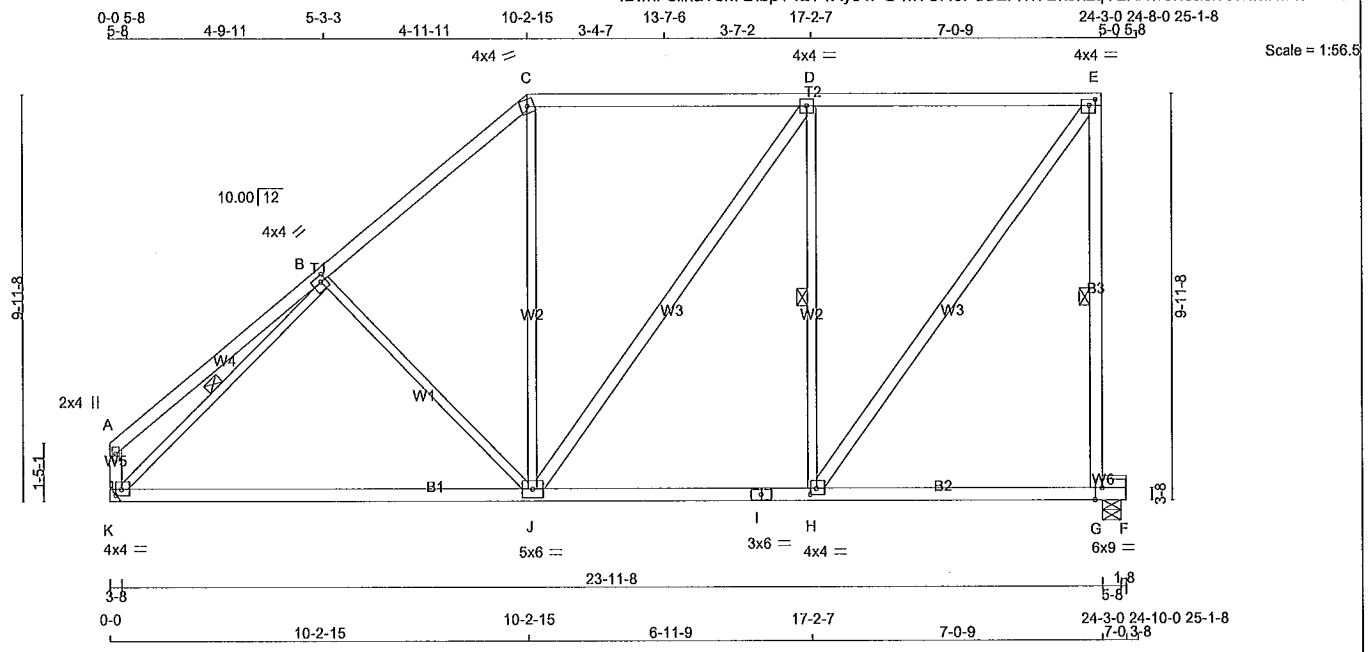


TOTAL WEIGHT = 7 X 125 = 877 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
K - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
[illegible]	2x4	DRY	No.2	SPF
[illegible]	2x4	DRY	No.2	SPF

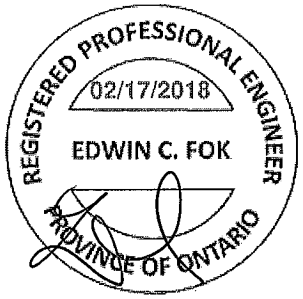
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0		
B	TMWW-t	MT20	4.0	4.0	1.75	1.50
C	TTW-m	MT20	4.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TMVW-t	MT20	4.0	4.0	1.75	1.75
G	BMVW1-t	MT20	6.0	9.0	Edge	2.00
H	BMWW-t	MT20	4.0	4.0	1.75	1.75
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	5.0	6.0		
K	BMVW1-t	MT20	4.0	4.0	1.75	1.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
K	1193	0	1193	0	0	IN-SX	IN-SX
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	
G	1307	0	1307	0	0	5-8	1-15

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	853	506 / 0	0 / 0	0 / 0	0 / 0	347 / 0	0 / 0
G	938	537 / 0	0 / 0	0 / 0	0 / 0	402 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

FOR SECTION C-E, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.71 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 5.87 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF B-K, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
K- A	-153 / 0	0.0	0.0	0.02 (1)	7.81
A- B	0 / 30	-78.0	-78.0	0.34 (1)	10.00
B- C	-1018 / 0	-78.0	-78.0	0.38 (1)	5.71
C- D	-759 / 0	-85.5	-85.5	0.75 (1)	2.00
D- E	-685 / 0	-85.5	-85.5	0.74 (1)	2.00
K- J	0 / 919	-18.5	-18.5	0.50 (4)	10.00
J- I	0 / 685	-18.5	-18.5	0.49 (4)	10.00
I- H	0 / 685	-18.5	-18.5	0.49 (4)	10.00
H- G	0 / 0	-18.5	-18.5	0.19 (4)	10.00
G- F	0 / 0	-96.5	-96.5	0.01 (1)	10.00
G- E	-1182 / 0	0.0	0.0	0.57 (1)	5.87

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- C	0 / 255	0.06 (4)
K- B	-1330 / 0	0.49 (1)
B- J	-234 / 0	0.26 (1)
J- D	0 / 128	0.02 (1)
H- D	-840 / 0	0.56 (1)
H- E	0 / 1180	0.19 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.80")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.80")
CALCULATED VERT. DEFL.(TL) = L/ 860 (0.34")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/0 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 0 (0.00")
ALLOWABLE DEFL.(TL)= L/0 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 0 (0.00")

CSI: TC=0.75/1.00 (C-D:1) , BC=0.57/1.00 (E-G:1) , WB=0.56/1.00 (D-H:1) , SSI=0.29/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (E) (INPUT = 0.90)
JSI METAL= 0.49 (B) (INPUT = 1.00)

A-18023496

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292593	H9T	7	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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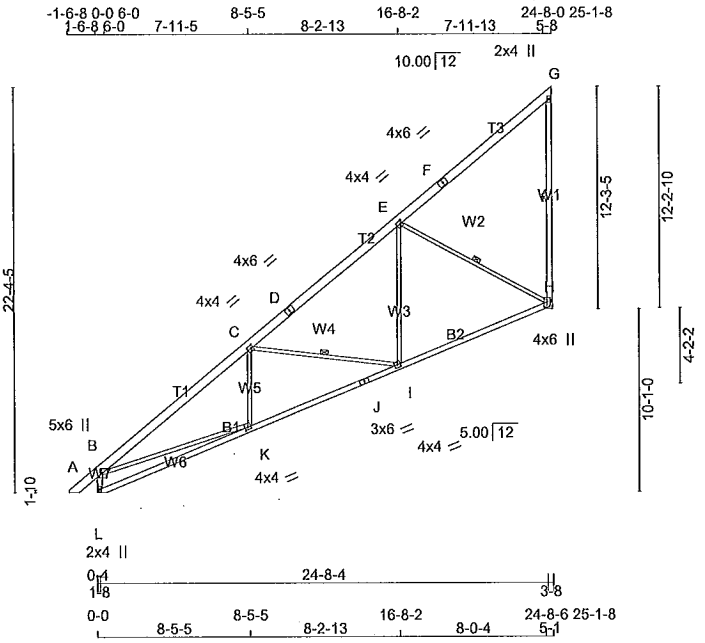


TOTAL WEIGHT = 7 X 138 = 969 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
L - B	2x4	DRY	No.2	SPF
A - D	2x6	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
F - G	2x6	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

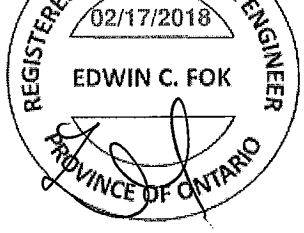
SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	3.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TS-t	MT20	4.0	6.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.25
F	TS-t	MT20	4.0	6.0		
G	TMV+p	MT20	2.0	4.0		
H	BVMW1+p	MT20	4.0	6.0	3.25	2.00
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0	1.50	2.00
L	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H	1212	0	1212	0	0	IN-SX	IN-SX
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	
L	1326	0	1326	0	0	HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	864	527 / 0	0 / 0	0 / 0	0 / 0	337 / 0	0 / 0
L	943	589 / 0	0 / 0	0 / 0	0 / 0	354 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.29 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-H, C-I, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- B	-1260 / 0	0.0	0.0	0.13 (1)	7.16	B- K	0 / 1639	0.37 (1)
A- B	0 / 36	-78.0	-78.0	0.06 (1)	10.00	K- C	-51 / 104	0.04 (4)
B- C	-2018 / 0	-78.0	-78.0	0.37 (1)	5.29	C- I	-666 / 0	0.32 (1)
C- D	-1192 / 0	-78.0	-78.0	0.30 (1)	6.25	I- E	0 / 545	0.12 (1)
D- E	-1192 / 0	-78.0	-78.0	0.30 (1)	6.25	E- H	-1047 / 0	0.67 (1)
E- F	-42 / 0	-78.0	-78.0	0.31 (1)	6.25			
F- G	-42 / 0	-78.0	-78.0	0.31 (1)	6.25			
H- G	-264 / 0	0.0	0.0	0.22 (1)	6.25			
L- K	0 / 6	-18.5	-18.5	0.29 (4)	10.00			
K- J	0 / 1710	-18.5	-18.5	0.46 (4)	10.00			
J- I	0 / 1710	-18.5	-18.5	0.46 (4)	10.00			
I- H	0 / 990	-18.5	-18.5	0.39 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.84")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.84")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.37/1.00 (B-C:1) , BC=0.46/1.00 (I-K:4) , WB=0.67/1.00 (E-H:1) , SSI=0.18/1.00 (E-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (K) (INPUT = 0.90)
JSI METAL= 0.62 (K) (INPUT = 1.00)

A-18023495

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292593	H8T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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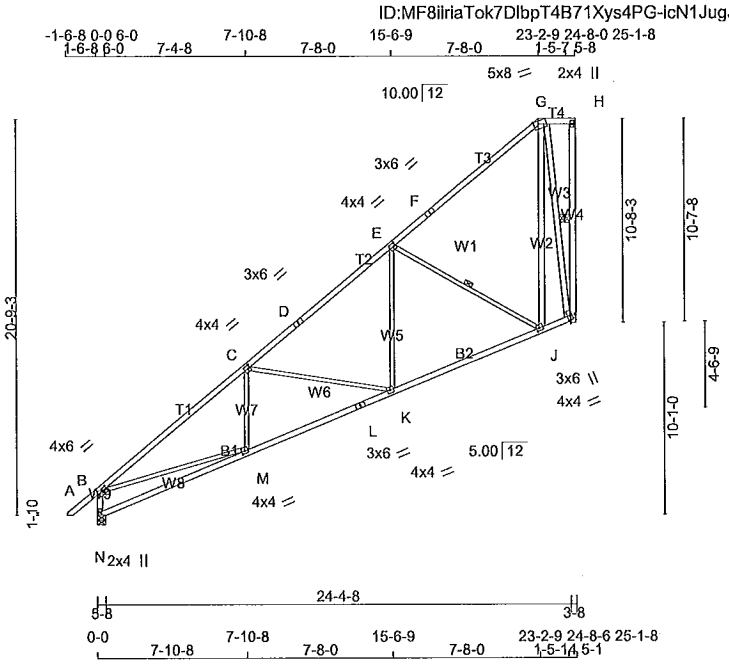


TOTAL WEIGHT = 136 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	2100F 1.8E	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
WEBS	2x3	DRY	No.2	SPF
JT G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF

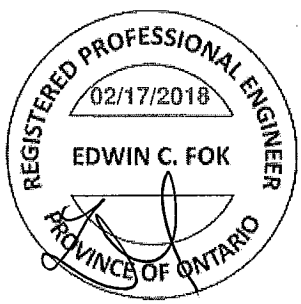
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	1.50	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.25
F	TS-t	MT20	3.0	6.0		
G	TTWW-m	MT20	5.0	8.0	Edge	3.00
H	TMV+p	MT20	2.0	4.0		
I	BVMW1+I	MT20	3.0	6.0	3.00	1.75
J	BMWW-t	MT20	4.0	4.0	1.75	1.50
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	4.0	1.50	2.00
N	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
I	1212	0	1212	0	0	IN-SX	IN-SX
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	
N	1330	0	1330	0	0	5-8	1-13

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	864	527 / 0	0 / 0	0 / 0	0 / 0	337 / 0	0 / 0
N	946	591 / 0	0 / 0	0 / 0	0 / 0	355 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.96 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF H-I, E-J, G-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 38	-78.0	-78.0	0.08 (1)	10.00	M- C	-93 / 82	0.03 (1)
B- C	-1996 / 0	-78.0	-78.0	0.50 (1)	5.22	C- K	-573 / 0	0.73 (1)
C- D	-1297 / 0	-78.0	-78.0	0.61 (1)	4.96	K- E	0 / 481	0.11 (1)
D- E	-1297 / 0	-78.0	-78.0	0.61 (1)	4.96	E- J	-932 / 0	0.53 (1)
E- F	-284 / 0	-78.0	-78.0	0.57 (1)	6.25	J- G	0 / 913	0.15 (1)
F- G	-284 / 0	-78.0	-78.0	0.57 (1)	6.25	G- I	-1088 / 0	0.64 (1)
G- H	0 / 0	-78.0	-78.0	0.05 (1)	10.00	B- M	0 / 1612	0.36 (1)
I- H	-74 / 0	0.0	0.0	0.05 (1)	6.25			
N- B	-1269 / 0	0.0	0.0	0.13 (1)	7.14			
N- M	0 / 6	-18.5	-18.5	0.27 (4)	10.00			
M- L	0 / 1690	-18.5	-18.5	0.43 (4)	10.00			
L- K	0 / 1690	-18.5	-18.5	0.43 (4)	10.00			
K- J	0 / 1077	-18.5	-18.5	0.31 (4)	10.00			
J- I	0 / 205	-18.5	-18.5	0.19 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.84")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.84")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.61/1.00 (C-E:1) , BC=0.43/1.00 (K-M:4) , WB=0.73/1.00 (C-K:1) , SSI=0.22/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.61 (M) (INPUT = 1.00)

A-18023494

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292593	H7T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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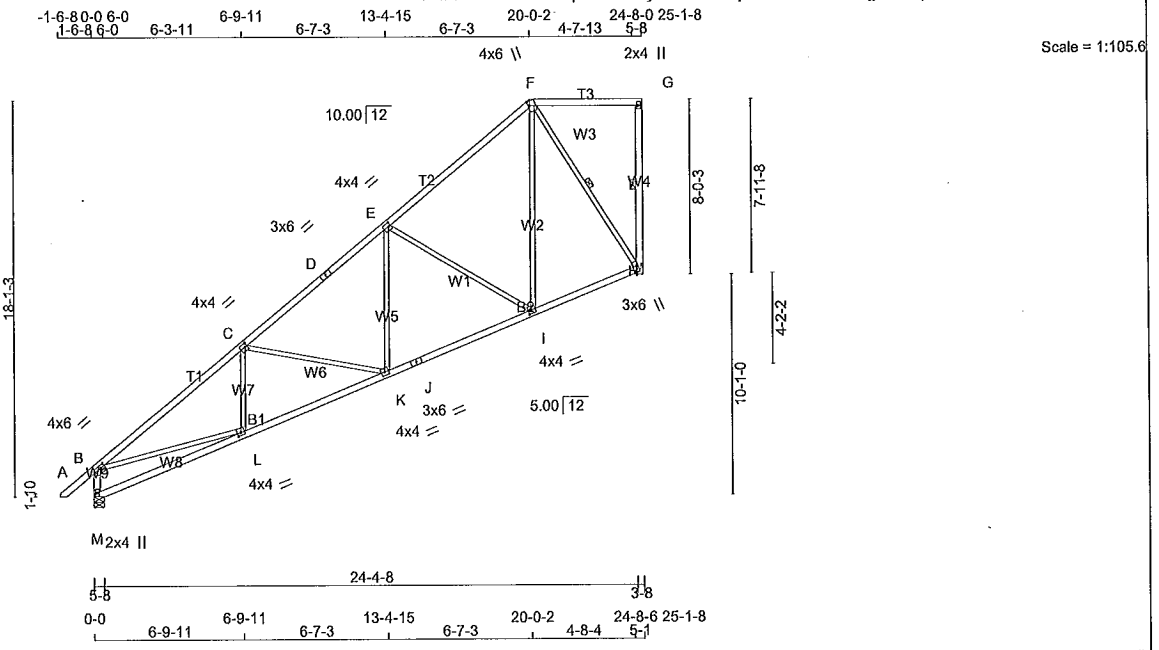


TOTAL WEIGHT = 119 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

PT

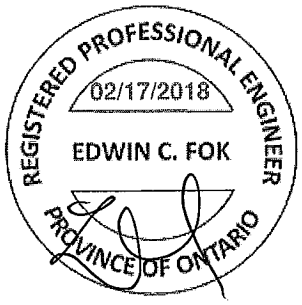
SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	1.50	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.25
F	TTWW+m	MT20	4.0	6.0	2.50	0.75
G	TMV+p	MT20	2.0	4.0		
H	BVMW1+I	MT20	3.0	6.0	Edge	2.00
I	BMWW-t	MT20	4.0	4.0	2.00	1.50
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMWW-t	MT20	4.0	4.0	1.50	2.00
M	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H	1212	0	1212	0	0	IN-SX	IN-SX
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	
M	1330	0	1330	0	0	5-8	1-13

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	864	527 / 0	0 / 0	0 / 0	0 / 0	337 / 0	0 / 0
M	946	591 / 0	0 / 0	0 / 0	0 / 0	355 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.22 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-H, F-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 38	-78.0	-78.0	0.13 (1)	10.00	L- C	-154 / 42	0.05 (1)
B- C	-2004 / 0	-78.0	-78.0	0.61 (1)	4.22	C- K	-409 / 0	0.35 (1)
C- D	-1511 / 0	-78.0	-78.0	0.45 (1)	4.90	K- E	0 / 371	0.08 (1)
D- E	-1511 / 0	-78.0	-78.0	0.45 (1)	4.90	E- I	-761 / 0	0.98 (1)
E- F	-697 / 0	-78.0	-78.0	0.42 (1)	6.25	I- F	0 / 783	0.18 (1)
F- G	0 / 0	-78.0	-78.0	0.35 (1)	10.00	F- H	-922 / 0	0.52 (1)
H- G	-199 / 0	0.0	0.0	0.06 (1)	6.25	B- L	0 / 1605	0.36 (1)
M- B	-1277 / 0	0.0	0.0	0.14 (1)	7.13			
M- L	0 / 5	-18.5	-18.5	0.19 (4)	10.00			
L- K	0 / 1690	-18.5	-18.5	0.37 (1)	10.00			
K- J	0 / 1257	-18.5	-18.5	0.28 (1)	10.00			
J- I	0 / 1257	-18.5	-18.5	0.28 (1)	10.00			
I- H	0 / 557	-18.5	-18.5	0.20 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.84")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.84")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.61/1.00 (B-C:1) , BC=0.37/1.00 (K-L:1) , WB=0.98/1.00 (E-I:1) , SSI=0.19/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (L) (INPUT = 0.90)
JSI METAL= 0.61 (L) (INPUT = 1.00)

A-18023493

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292593	H6T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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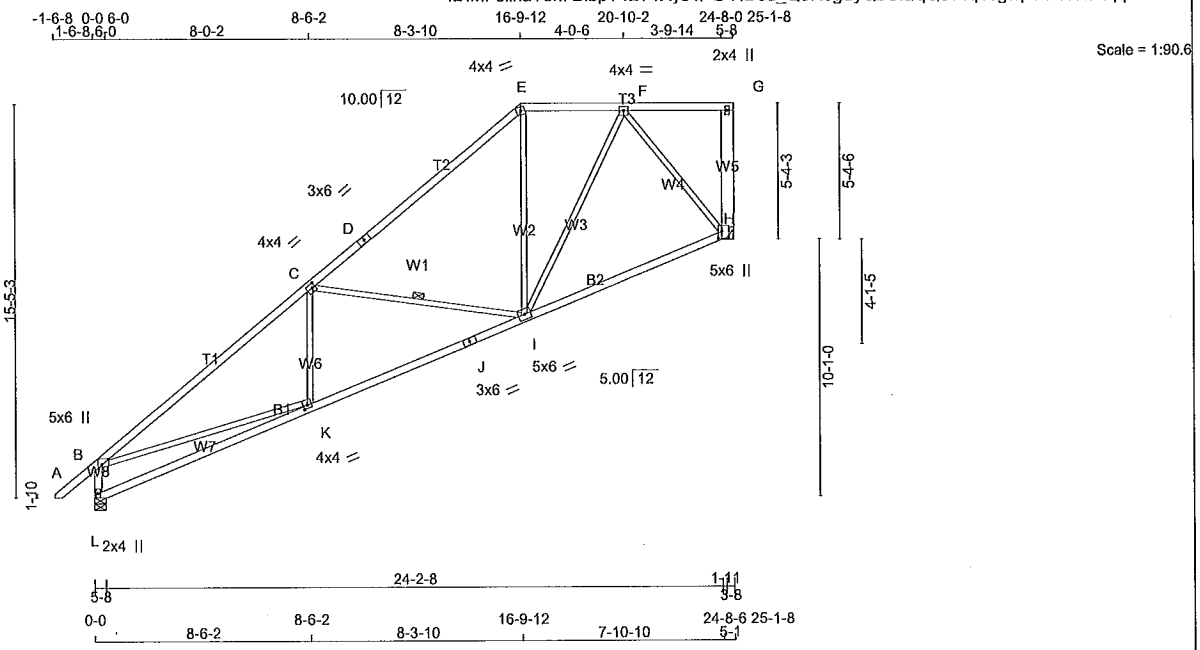


TOTAL WEIGHT = 114 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	2100F 1.8E	SPF
D - E	2x4	DRY	2100F 1.8E	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x6	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

PT

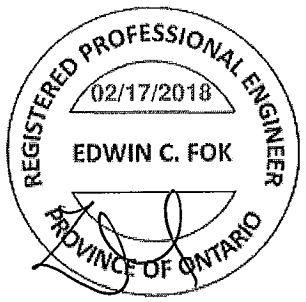
L SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	3.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TS-t	MT20	3.0	6.0		
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMV+p	MT20	2.0	4.0		
H	BVMW1+p	MT20	5.0	6.0	3.25	1.75
I	BMWWW-t	MT20	5.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0	1.50	2.00
L	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
L	1330	0	1330	0	0	5-8	1-13
H	1212	0	1212	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	946	591 / 0	0 / 0	0 / 0	0 / 0	355 / 0	0 / 0
H	864	527 / 0	0 / 0	0 / 0	0 / 0	337 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 38	-78.0	-78.0	0.08 (1)	10.00	K- C	-65 / 101	0.04 (4)
B- C	-2005 / 0	-78.0	-78.0	0.65 (1)	5.00	C- I	-768 / 0	0.39 (1)
C- D	-1120 / 0	-78.0	-78.0	0.54 (1)	6.25	I- E	0 / 321	0.07 (1)
D- E	-1120 / 0	-78.0	-78.0	0.54 (1)	6.25	B- K	0 / 1634	0.37 (1)
E- F	-823 / 0	-78.0	-78.0	0.18 (1)	6.25	I- F	0 / 387	0.09 (1)
F- G	0 / 0	-78.0	-78.0	0.22 (1)	10.00	F- H	-996 / 0	0.74 (1)
H- G	-129 / 0	0.0	0.0	0.05 (1)	7.81			
L- B	-1264 / 0	0.0	0.0	0.13 (1)	7.16			
L- K	0 / 6	-18.5	-18.5	0.31 (4)	10.00			
K- J	0 / 1709	-18.5	-18.5	0.47 (4)	10.00			
J- I	0 / 1709	-18.5	-18.5	0.47 (4)	10.00			
I- H	0 / 707	-18.5	-18.5	0.35 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.84")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.84")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.65/1.00 (B-C:1) , BC=0.47/1.00 (I-K:4) , WB=0.74/1.00 (F-H:1) , SSI=0.25/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL= 0.62 (K) (INPUT = 1.00)

A-18023492

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292593	H5T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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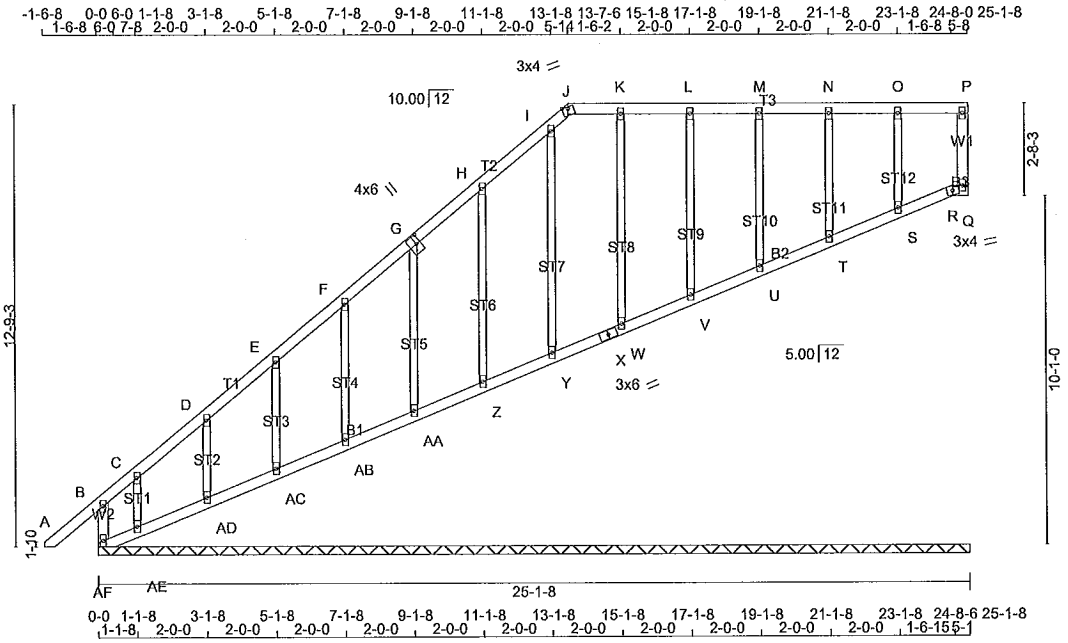


TOTAL WEIGHT = 112 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
J - P	2x4	DRY	No.2	SPF
Q - P	2x4	DRY	No.2	SPF
AF- B	2x4	DRY	No.2	SPF
AF- X	2x4	DRY	No.2	SPF
X - R	2x4	DRY	No.2	SPF
R - Q	2x4	DRY	No.2	SPF
WEBS	2x3	DRY	No.2	SPF
BRACLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

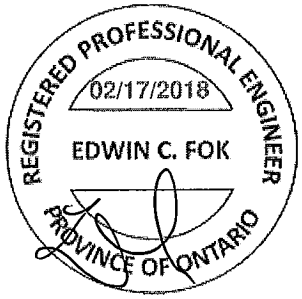
GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C, D, E, F, H, I, K, L, M, N, O						
C	TMW+w	MT20	2.0	4.0		
G	TSW+l	MT20	4.0	6.0	3.00	1.50
J	TT-m	MT20	3.0	4.0	Edge	
P	TMV+p	MT20	2.0	4.0		
Q	BMV1+p	MT20	2.0	4.0		
R	BB1-m	MT20	3.0	4.0		
S, T, U, V, W, Y, Z, AA, AB, AC, AD, AE						
S	BMW1+w	MT20	2.0	4.0		
X	BS-t	MT20	3.0	6.0		
AF	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): Q , R , S , T , U , V , W
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 38	-78.0	-78.0	0.13 (1)	10.00	S- O	-177 / 0	0.03 (1)
B- C	-102 / 0	-78.0	-78.0	0.13 (1)	6.25	T- N	-152 / 0	0.04 (1)
C- D	-35 / 0	-78.0	-78.0	0.04 (1)	6.25	U- M	-157 / 0	0.06 (1)
D- E	-34 / 0	-78.0	-78.0	0.04 (1)	6.25	V- L	-155 / 0	0.08 (1)
E- F	-19 / 0	-78.0	-78.0	0.05 (1)	6.25	W- K	-161 / 0	0.12 (1)
F- G	-25 / 0	-78.0	-78.0	0.05 (1)	6.25	Y- I	-152 / 0	0.13 (1)
G- H	-2 / 0	-78.0	-78.0	0.04 (1)	10.00	Z- H	-167 / 0	0.11 (1)
H- I	-8 / 0	-78.0	-78.0	0.04 (1)	10.00	AA- G	-123 / 0	0.05 (1)
I- J	-40 / 0	-78.0	-78.0	0.03 (1)	6.25	AB- F	-176 / 0	0.06 (1)
J- K	-2 / 0	-78.0	-78.0	0.04 (1)	10.00	AC- E	-141 / 0	0.03 (1)
K- L	0 / 0	-78.0	-78.0	0.04 (1)	10.00	AD- D	-166 / 0	0.03 (1)
L- M	0 / 1	-78.0	-78.0	0.04 (1)	10.00	AE- C	-14 / 0	0.00 (1)
M- N	0 / 3	-78.0	-78.0	0.04 (1)	10.00			
N- O	0 / 5	-78.0	-78.0	0.04 (1)	10.00			
O- P	0 / 9	-78.0	-78.0	0.04 (1)	10.00			
Q- P	-61 / 0	0.0	0.0	0.02 (1)	7.81			
AF- B	-276 / 0	0.0	0.0	0.06 (1)	7.81			
AF-AE	0 / 24	-18.5	-18.5	0.06 (1)	10.00			
AE-AD	0 / 39	-18.5	-18.5	0.02 (4)	10.00			
AD-AC	0 / 25	-18.5	-18.5	0.02 (4)	10.00			
AC-AB	0 / 19	-18.5	-18.5	0.02 (4)	10.00			
AB-AA	0 / 12	-18.5	-18.5	0.02 (4)	10.00			
AA- Z	0 / 10	-18.5	-18.5	0.02 (4)	10.00			
Z- Y	0 / 6	-18.5	-18.5	0.01 (4)	10.00			
Y- X	0 / 2	-18.5	-18.5	0.01 (4)	10.00			
X- W	0 / 2	-18.5	-18.5	0.01 (4)	10.00			
W- V	0 / 0	-18.5	-18.5	0.01 (4)	10.00			
V- U	-1 / 0	-18.5	-18.5	0.01 (4)	10.00			
U- T	-3 / 0	-18.5	-18.5	0.02 (4)	10.00			
T- S	-6 / 0	-18.5	-18.5	0.02 (4)	10.00			
S- R	-9 / 0	-18.5	-18.5	0.01 (4)	10.00			
R- Q	-8 / 0	-18.5	-18.5	0.02 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.13/1.00 (B-C:1) , BC=0.06/1.00 (AE-AF:1) , WB=0.13/1.00 (I-Y:1) , SSI=0.08/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.52 (G) (INPUT = 0.90)
JSI METAL= 0.17 (B) (INPUT = 1.00)

A-18023491

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292593	G6P	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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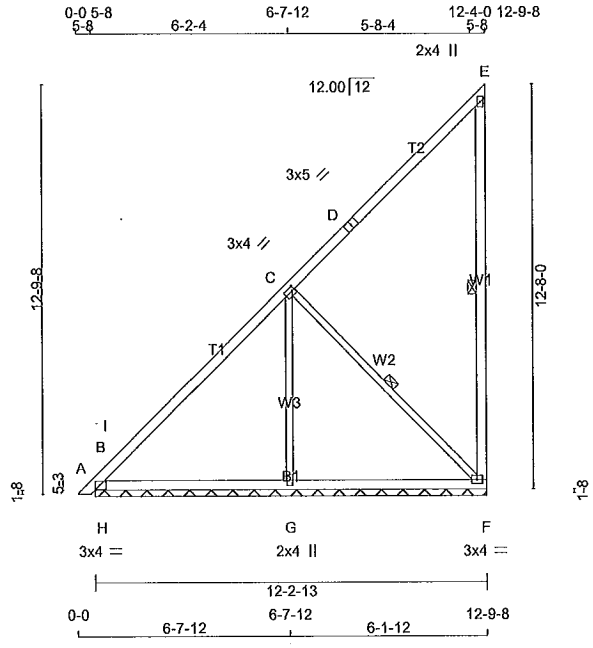


TOTAL WEIGHT = 60 lb

[M]

LUMBER

N. L. G. A. RULES

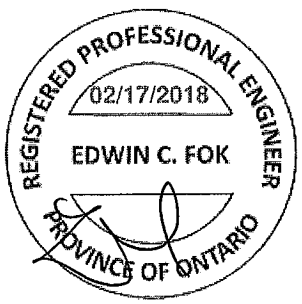
CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.25
C	TMWW-t	MT20	3.0	4.0	1.50	1.00
D	TS-t	MT20	3.0	5.0		
E	TMV+p	MT20	2.0	4.0		
F	BMVW1-t	MT20	3.0	4.0		
G	BMW1+w	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	373	0	373	0	0	12-2-13	2-0-2
B	406	0	406	0	0	12-2-13	2-0-2
G	436	0	436	0	0	12-2-13	2-0-2

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	264	174 / 0	0 / 0	0 / 0	0 / 0	90 / 0	0 / 0
B	286	192 / 0	0 / 0	0 / 0	0 / 0	94 / 0	0 / 0
G	315	166 / 0	0 / 0	0 / 0	0 / 0	149 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, C-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 11	-78.0	-78.0	0.01 (1)	10.00	G- C	-270 / 0	0.19 (1)
B- I	-77 / 31	-78.0	-78.0	0.10 (1)	6.25	C- F	-211 / 0	0.11 (1)
I- C	-179 / 0	-78.0	-78.0	0.47 (1)	6.25	H- I	-487 / 0	0.00 (1)
C- D	-39 / 0	-78.0	-78.0	0.46 (1)	6.25			
D- E	-39 / 0	-78.0	-78.0	0.46 (1)	6.25			
F- E	-185 / 0	0.0	0.0	0.16 (1)	6.25			
B- H	0 / 149	-18.5	-18.5	0.16 (1)	10.00			
H- G	0 / 149	-18.5	-18.5	0.21 (4)	10.00			
G- F	0 / 149	-18.5	-18.5	0.21 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.47/1.00 (C-I:1) , BC=0.21/1.00 (G-H:4) , WB=0.19/1.00 (C-G:1) , SSI=0.40/1.00 (B-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.65 (D) (INPUT = 0.90)
JSI METAL= 0.21 (D) (INPUT = 1.00)

A-18023490

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292593	G5P	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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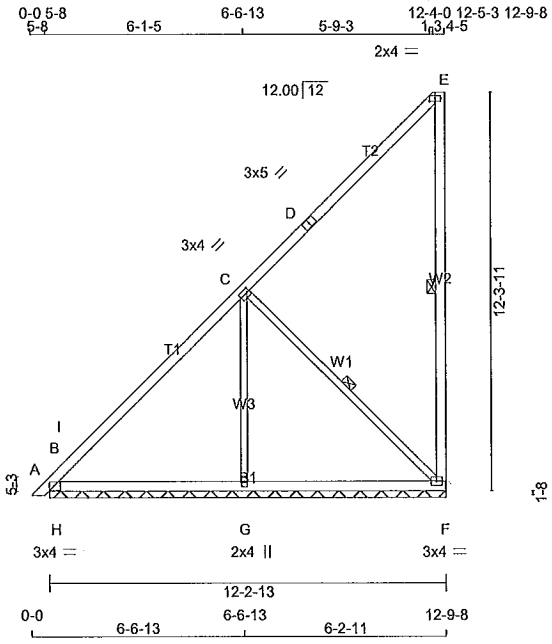


Scale = 1:71.2

TOTAL WEIGHT = 59 lb

[M]

LUMBER

N. L. G. A. RULES

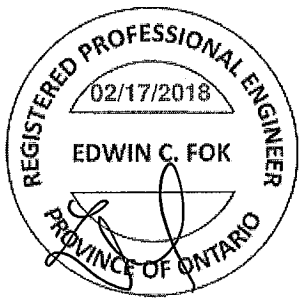
CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

ES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.25
C	TMWW-t	MT20	3.0	4.0	1.50	1.00
D	TS-t	MT20	3.0	5.0		
E	TVM-p	MT20	2.0	4.0	0.75	2.00
F	BMVW1-t	MT20	3.0	4.0		
G	BMW1+w	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	373	0	373	0	0	12-2-13	2-0-7
B	400	0	400	0	0	12-2-13	2-0-7
G	442	0	442	0	0	12-2-13	2-0-7

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	264	174 / 0	0 / 0	0 / 0	0 / 0	90 / 0	0 / 0
B	282	189 / 0	0 / 0	0 / 0	0 / 0	93 / 0	0 / 0
G	319	169 / 0	0 / 0	0 / 0	0 / 0	150 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, C-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 11	-78.0	-78.0	0.01 (1)	10.00	G- C	-277 / 0	0.19 (1)
B- I	-76 / 15	-78.0	-78.0	0.09 (1)	6.25	C- F	-207 / 0	0.10 (1)
I- C	-176 / 0	-78.0	-78.0	0.47 (1)	6.25	H- I	-460 / 0	0.00 (1)
C- D	-38 / 0	-78.0	-78.0	0.46 (1)	6.25			
D- E	-38 / 0	-78.0	-78.0	0.46 (1)	6.25			
F- E	-188 / 0	0.0	0.0	0.17 (1)	6.25			
B- H	0 / 148	-18.5	-18.5	0.16 (1)	10.00			
H- G	0 / 148	-18.5	-18.5	0.21 (4)	10.00			
G- F	0 / 148	-18.5	-18.5	0.21 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.47/1.00 (C-I:1) , BC=0.21/1.00 (G-H:4) , WB=0.19/1.00 (C-G:1) , SSI=0.38/1.00 (B-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (D) (INPUT = 0.90)
JSI METAL= 0.25 (D) (INPUT = 1.00)

A-18023489

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292593	G4P	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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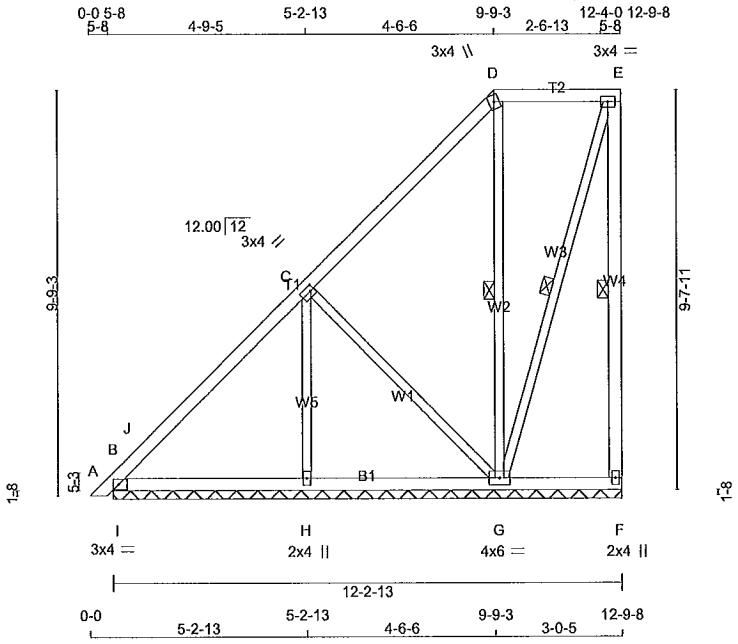


Scale = 1:55.5

TOTAL WEIGHT = 67 lb

[M]

LUMBER

N. L. G. A. RULES

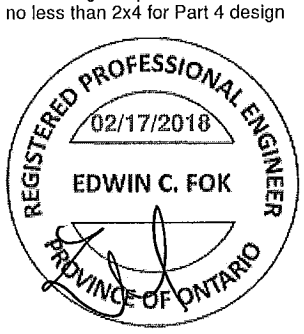
CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

:S (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.25
C	TMWW-t	MT20	3.0	4.0	1.50	1.00
D	TTW+m	MT20	3.0	4.0		
E	TMVW-t	MT20	3.0	4.0		
F	BMV1+p	MT20	2.0	4.0		
G	BMWWW1-t	MT20	4.0	6.0		
H	BMW1+w	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	123	0	123	0	0	12-2-13	1-6-3
B	315	0	315	0	0	12-2-13	1-6-3
H	329	0	329	0	0	12-2-13	1-6-3
G	449	0	449	0	0	12-2-13	1-6-3

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	87	55 / 0	0 / 0	0 / 0	0 / 0	32 / 0	0 / 0
B	222	148 / 0	0 / 0	0 / 0	0 / 0	74 / 0	0 / 0
H	237	129 / 0	0 / 0	0 / 0	0 / 0	108 / 0	0 / 0
G	319	200 / 0	0 / 0	0 / 0	0 / 0	119 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, D-G, E-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 11	-78.0	-78.0	0.01 (1)	10.00	H- C	-208 / 0	0.08 (1)
B- J	-63 / 0	-78.0	-78.0	0.05 (1)	6.25	C- G	-155 / 0	0.13 (1)
J- C	-126 / 0	-78.0	-78.0	0.26 (1)	6.25	G- D	-255 / 0	0.16 (1)
C- D	-27 / 0	-78.0	-78.0	0.26 (1)	6.25	G- E	-16 / 0	0.01 (1)
D- E	0 / 5	-78.0	-78.0	0.12 (1)	10.00	I- J	-298 / 1	0.00 (1)
F- E	-102 / 0	0.0	0.0	0.05 (1)	6.25			
B- I	0 / 106	-18.5	-18.5	0.10 (1)	10.00			
I- H	0 / 106	-18.5	-18.5	0.11 (1)	10.00			
H- G	0 / 106	-18.5	-18.5	0.11 (4)	10.00			
G- F	0 / 0	-18.5	-18.5	0.06 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.26/1.00 (C-J:1) , BC=0.11/1.00 (H-I:1) , WB=0.16/1.00 (D-G:1) , SSI=0.25/1.00 (B-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.30 (B) (INPUT = 0.90)
JSI METAL= 0.08 (D) (INPUT = 1.00)

A-18023488

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292593	G3P	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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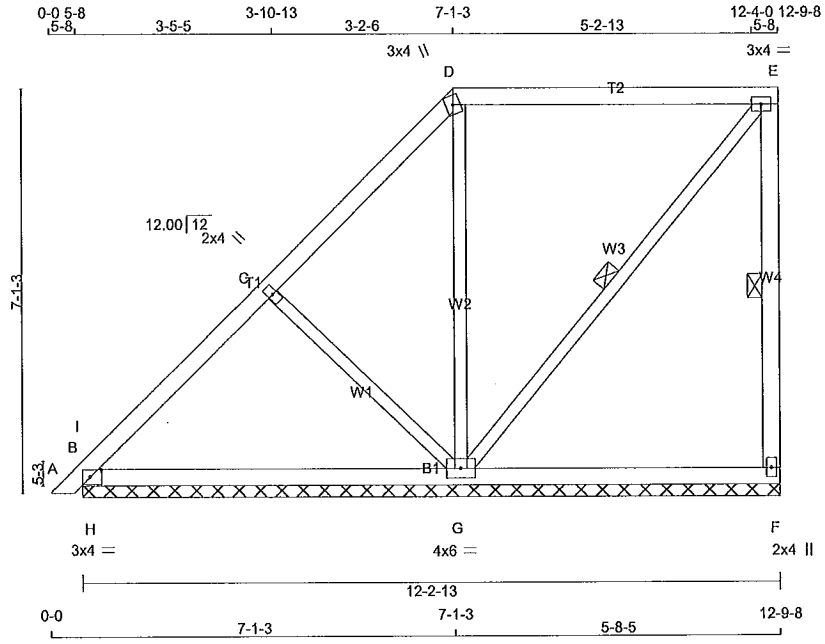


Scale = 1:40.6

TOTAL WEIGHT = 55 lb [M]

LUMBER
N. L. G. A. RULES

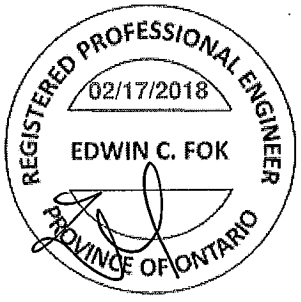
CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-t	MT20	3.0	4.0	1.50	2.25
C	TMW+w	MT20	2.0	4.0		
D	TTW+m	MT20	3.0	4.0		
E	TMVW-t	MT20	3.0	4.0		
F	BMV1+p	MT20	2.0	4.0		
G	BMWWW1-t	MT20	4.0	6.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	249	0	249	0	0	12-2-13	1-2-11
B	338	0	338	0	0	12-2-13	1-2-11
G	628	0	628	0	0	12-2-13	1-2-11

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	177	111 / 0	0 / 0	0 / 0	0 / 0	66 / 0	0 / 0
B	240	153 / 0	0 / 0	0 / 0	0 / 0	87 / 0	0 / 0
G	448	268 / 0	0 / 0	0 / 0	0 / 0	180 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, E-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 11	-78.0	-78.0	0.01 (1)	10.00	C- G	-228 / 0	0.08 (1)
B- I	-372 / 0	-78.0	-78.0	0.08 (4)	6.25	G- D	-321 / 0	0.27 (1)
I- C	-196 / 0	-78.0	-78.0	0.15 (1)	6.25	G- E	-13 / 3	0.01 (1)
C- D	-19 / 0	-78.0	-78.0	0.14 (1)	6.25	H- I	0 / 211	0.00 (1)
D- E	-2 / 8	-78.0	-78.0	0.43 (1)	10.00			
F- E	-212 / 0	0.0	0.0	0.05 (1)	6.25			
B- H	0 / 160	-18.5	-18.5	0.08 (4)	10.00			
H- G	0 / 160	-18.5	-18.5	0.19 (4)	10.00			
G- F	0 / 0	-18.5	-18.5	0.18 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.43/1.00 (D-E:1) , BC=0.19/1.00 (G-H:4) , WB=0.27/1.00 (D-G:1) , SSI=0.18/1.00 (B-H:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.50 (D) (INPUT = 0.90)
JSI METAL= 0.11 (D) (INPUT = 1.00)

A-18023487

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292593	G2P	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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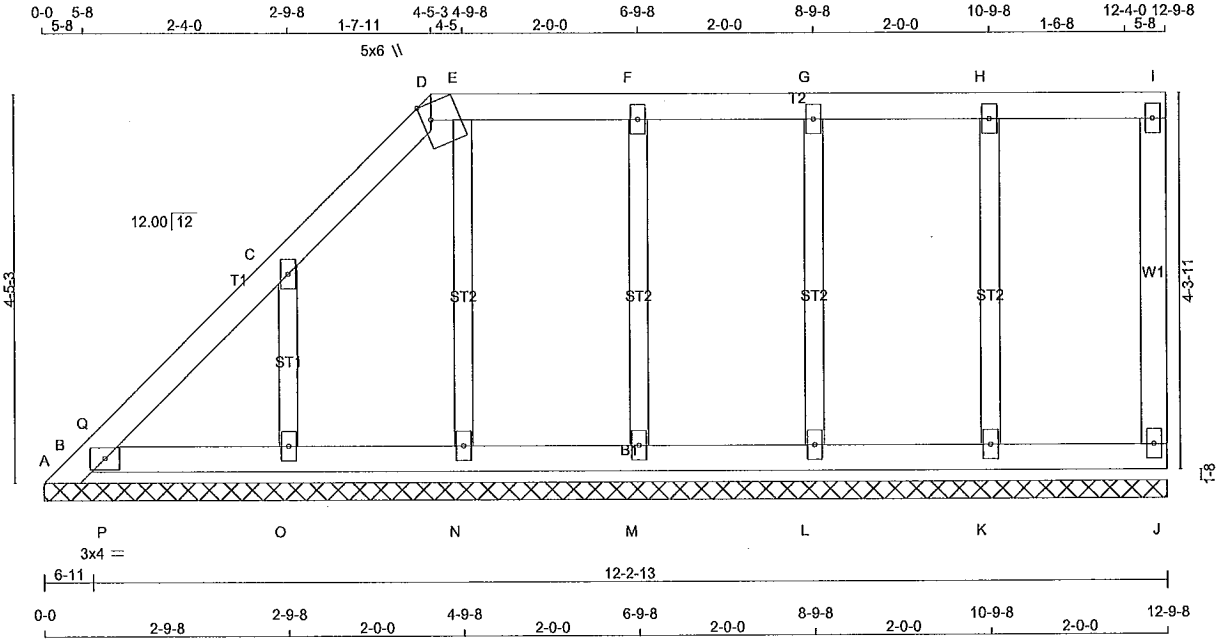


TOTAL WEIGHT = 49 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - I	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
B - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

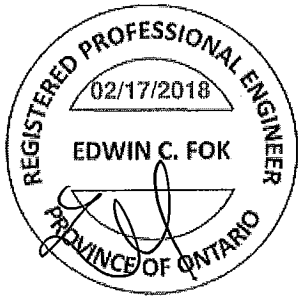
: STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C, F, G, H						
C	TMW+w	MT20	2.0	4.0		
D						
D	TTW+m	MT20	5.0	6.0	Edge	1.25
E						
I	TMV+p	MT20	2.0	4.0		
J	BMV1+p	MT20	2.0	4.0		
K, L, M, N, O						
K	BMW1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): J , K , L , M , N , O

PROVIDE ANCHORAGE AT BEARING JOINT A FOR 150 LBS FACTORED UPLIFT
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S).

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 31	-96.5	-96.5	0.03 (1)	10.00
B- Q	-53 / 0	-78.0	-78.0	0.03 (1)	6.25
Q- C	-4 / 0	-78.0	-78.0	0.04 (1)	10.00
C- D	-12 / 0	-78.0	-78.0	0.04 (1)	6.25
D- E	0 / 5	-78.0	-78.0	0.03 (1)	10.00
E- F	0 / 0	-78.0	-78.0	0.04 (1)	10.00
F- G	0 / 0	-78.0	-78.0	0.04 (1)	10.00
G- H	0 / 0	-78.0	-78.0	0.04 (1)	10.00
H- I	0 / 0	-78.0	-78.0	0.04 (1)	10.00
J- I	-63 / 0	0.0	0.0	0.02 (1)	7.81
B- P	0 / 8	-18.5	-18.5	0.01 (1)	10.00
P- O	0 / 8	-18.5	-18.5	0.02 (4)	10.00
O- N	0 / 0	-18.5	-18.5	0.02 (4)	10.00
N- M	0 / 0	-18.5	-18.5	0.02 (4)	10.00
M- L	0 / 0	-18.5	-18.5	0.02 (4)	10.00
L- K	0 / 0	-18.5	-18.5	0.02 (4)	10.00
K- J	0 / 0	-18.5	-18.5	0.02 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- H	-174 / 0	0.05 (1)
L- G	-152 / 0	0.04 (1)
M- F	-159 / 0	0.04 (1)
N- E	-155 / 0	0.04 (1)
O- C	-163 / 0	0.03 (1)
P- Q	-21 / 24	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.04/1.00 (C-D:1) , BC=0.02/1.00 (O-P:4) , WB=0.05/1.00 (H-K:1) , SSI=0.07/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (C) (INPUT = 0.90)
JSI METAL= 0.09 (C) (INPUT = 1.00)

A-18023486

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292593	G1G	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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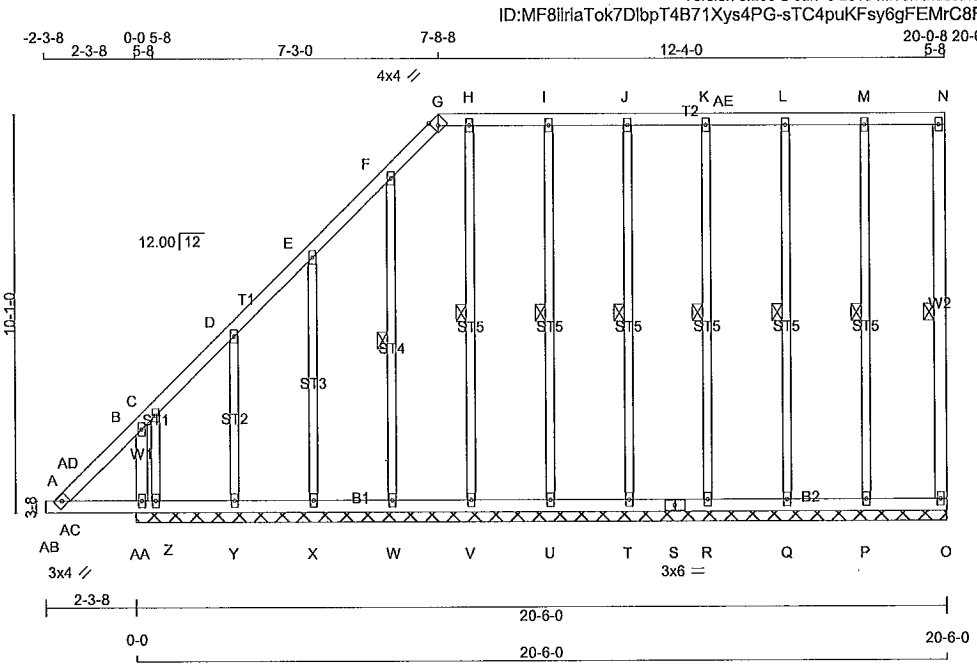


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
AA- B	2x4	DRY	No.2	SPF
A - G	2x4	DRY	No.2	SPF
G - N	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
AB- S	2x4	DRY	No.2	SPF
S - O	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

SEASONED LUMBER.

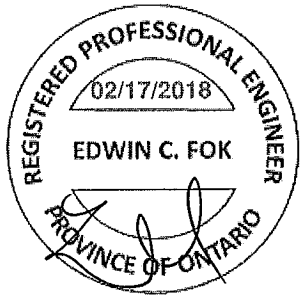
GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM-h	MT20	3.0	4.0		
B	TMV+p	MT20	2.0	4.0		
C, D, E, F, H, I, J, K, L, M						
C	TMW+w	MT20	2.0	4.0		
G	TT-h	MT20	4.0	4.0	Edge	1.50
N	TMV+p	MT20	2.0	4.0		
O	BMV1+p	MT20	2.0	4.0		
P, Q, R, T, U, V, W, X, Y, Z						
P	BMW1+w	MT20	2.0	4.0		
S	BS-t	MT20	3.0	6.0		
AA	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

PROVIDE ANCHORAGE AT BEARING JOINT Z FOR 382 LBS FACTORED UPLIFT
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

FOR SECTION G-N, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF N-O, M-P, L-Q, K-R, J-T, I-U, H-V, F-W.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
AA- B	-347 / 0	0.0	0.0	0.04 (1)	7.81	P- M	-187 / 0	0.13 (1)
A-AD	0 / 26	-78.0	-78.0	0.02 (1)	10.00	Q- L	-170 / 0	0.12 (1)
AD- B	0 / 31	-78.0	-78.0	0.14 (1)	10.00	R- K	-171 / 0	0.12 (1)
B- C	-155 / 0	-78.0	-78.0	0.14 (1)	6.25	T- J	-173 / 0	0.12 (1)
C- D	-8 / 0	-78.0	-78.0	0.04 (1)	10.00	U- I	-158 / 0	0.11 (1)
D- E	-7 / 0	-78.0	-78.0	0.04 (1)	10.00	V- H	-190 / 0	0.13 (1)
E- F	0 / 0	-78.0	-78.0	0.04 (1)	10.00	W- F	-156 / 0	0.08 (1)
F- G	-18 / 0	-78.0	-78.0	0.04 (1)	6.25	X- E	-148 / 0	0.12 (1)
G- H	0 / 2	-85.5	-85.5	0.07 (1)	2.00	Y- D	-159 / 0	0.06 (1)
H- I	0 / 0	-85.5	-85.5	0.06 (1)	10.00	Z- C	0 / 121	0.03 (1)
I- J	0 / 0	-85.5	-85.5	0.04 (1)	10.00	AC-AD	-80 / 0	0.00 (1)
J-AE	0 / 0	-85.5	-85.5	0.04 (1)	10.00			
AE- K	0 / 0	-85.5	-85.5	0.04 (1)	10.00			
K- L	0 / 0	-85.5	-85.5	0.04 (1)	10.00			
L- M	0 / 0	-85.5	-85.5	0.04 (1)	10.00			
M- N	0 / 0	-85.5	-85.5	0.04 (1)	10.00			
O- N	-70 / 0	0.0	0.0	0.03 (1)	6.25			
AB- A	0 / 0	-33.5	-33.5	0.00 (4)	10.00			
A-AC	-1 / 3	-18.5	-18.5	0.01 (1)	10.00			
AC-AA	-1 / 3	-18.5	-18.5	0.17 (1)	10.00			
AA- Z	-1 / 3	-18.5	-18.5	0.17 (1)	10.00			
Z- Y	0 / 8	-18.5	-18.5	0.02 (4)	10.00			
Y- X	0 / 4	-18.5	-18.5	0.02 (4)	10.00			
X- W	0 / 2	-18.5	-18.5	0.01 (4)	10.00			
W- V	0 / 0	-18.5	-18.5	0.01 (4)	10.00			
V- U	0 / 0	-18.5	-18.5	0.01 (4)	10.00			
U- T	0 / 0	-18.5	-18.5	0.01 (4)	10.00			
T- S	0 / 0	-18.5	-18.5	0.01 (4)	10.00			
S- R	0 / 0	-18.5	-18.5	0.01 (4)	10.00			
R- Q	0 / 0	-18.5	-18.5	0.01 (4)	10.00			
Q- P	0 / 0	-18.5	-18.5	0.02 (4)	10.00			
P- O	0 / 0	-18.5	-18.5	0.02 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/1.00 (B-AD:1) , BC=0.17/1.00 (AA-AC:1) , WB=0.13/1.00 (H-V:1) , SSI=0.22/1.00 (Z-AA:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.42 (G) (INPUT = 0.90)
JSI METAL= 0.18 (B) (INPUT = 1.00)

A-18023485

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292593	G1	4	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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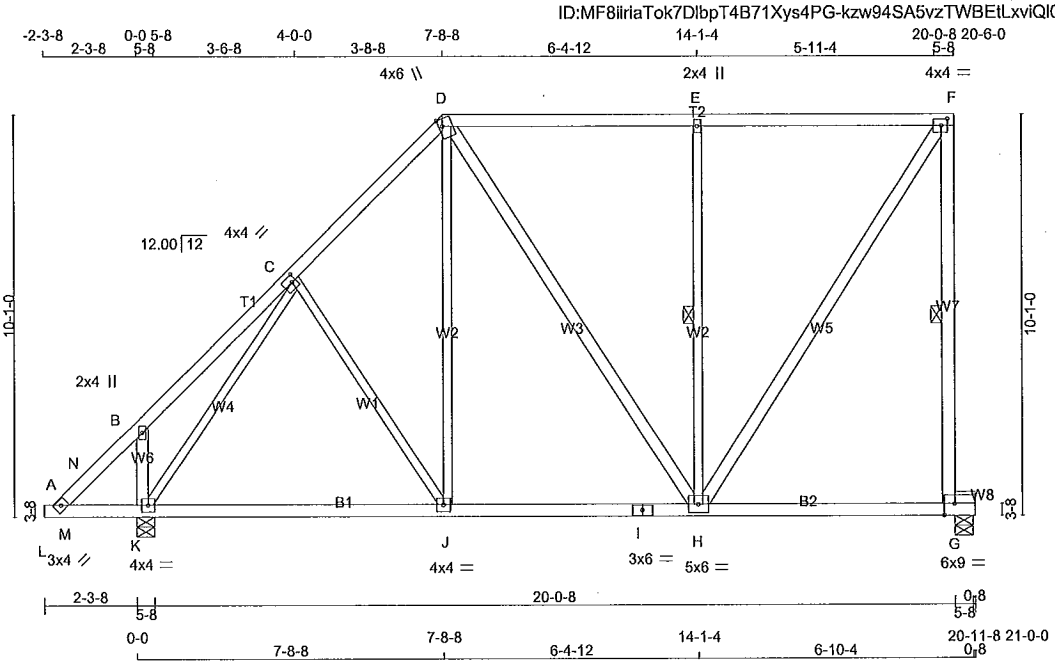


TOTAL WEIGHT = 4 X 123 = 491 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
	2x4	DRY	No.2	SPF

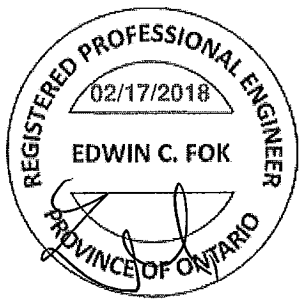
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM-h	MT20	3.0	4.0		
B	TMV+p	MT20	2.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	4.0	6.0	Edge	1.00
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	4.0	2.00	1.75
G	BMVW1-t	MT20	6.0	9.0	Edge	3.00
H	BMWWW-t	MT20	5.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1044	0	1044	0	0	5-8	1-9
K	1238	0	1238	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	750	426 / 0	0 / 0	0 / 0	0 / 0	324 / 0	0 / 0
K	886	522 / 0	0 / 0	0 / 0	0 / 0	363 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

FOR SECTION D-F, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- N	0 / 42	-78.0	-78.0	0.04 (4)	10.00	
N- B	-15 / 53	-78.0	-78.0	0.13 (1)	6.25	
B- C	-16 / 25	-78.0	-78.0	0.16 (1)	6.25	
C- D	-734 / 0	-78.0	-78.0	0.14 (1)	6.25	
D- E	-515 / 0	-85.5	-85.5	0.45 (1)	2.00	
E- F	-515 / 0	-85.5	-85.5	0.45 (1)	2.00	
G- F	-996 / 0	0.0	0.0	0.49 (1)	6.25	
K- B	-258 / 0	0.0	0.0	0.03 (1)	7.81	
L- A	0 / 0	-33.5	-33.5	0.00 (4)	10.00	
A- M	-15 / 12	-18.5	-18.5	0.02 (4)	6.25	
M- K	-15 / 12	-18.5	-18.5	0.21 (4)	6.25	
K- J	0 / 525	-18.5	-18.5	0.26 (4)	10.00	
J- I	0 / 504	-18.5	-18.5	0.23 (4)	10.00	
I- H	0 / 504	-18.5	-18.5	0.23 (4)	10.00	
H- G	0 / 0	-18.5	-18.5	0.17 (4)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- D	0 / 160	0.04 (4)
D- H	0 / 20	0.00 (1)
H- E	-678 / 0	0.47 (1)
H- F	0 / 942	0.15 (1)
K- C	-985 / 0	0.91 (1)
C- J	-40 / 21	0.04 (1)
M- N	-69 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/1319 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/1319 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.49/1.00 (F-G:1) , BC=0.26/1.00 (J-K:4) , WB=0.91/1.00 (C-K:1) , SSI=0.27/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (H) (INPUT = 0.90)
JSI METAL= 0.36 (C) (INPUT = 1.00)

A-18023484

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292590	J1TA	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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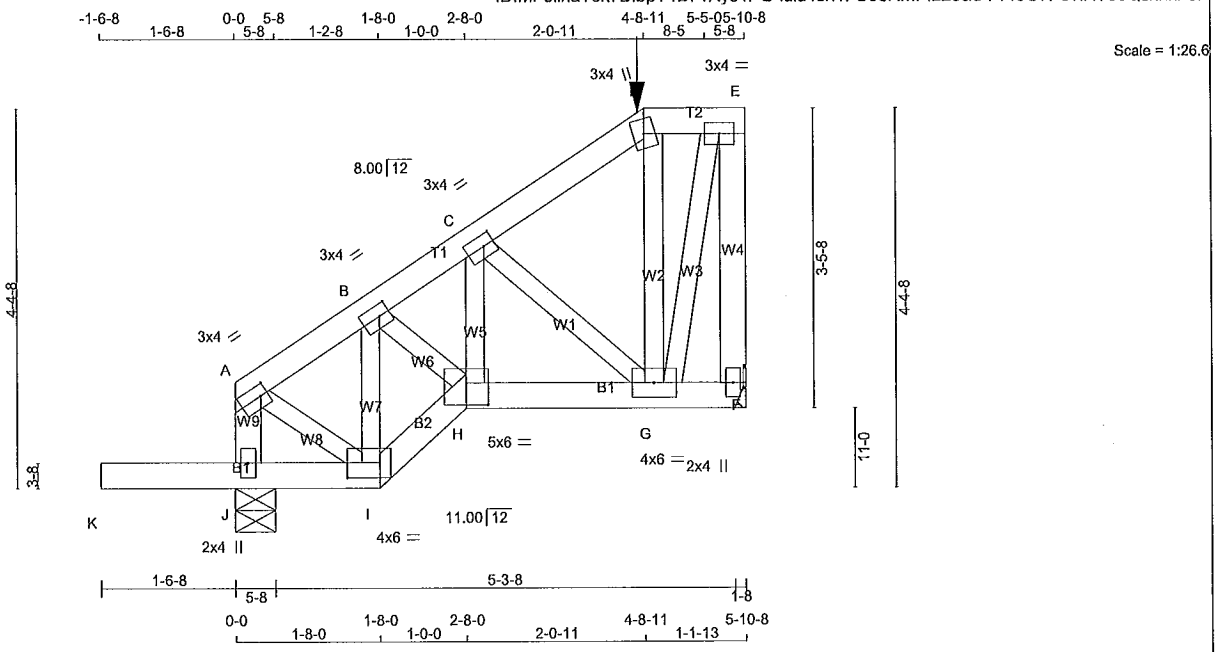


TOTAL WEIGHT = 2 X 33 = 66 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - A	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

F 2T

EASONED LUMBER.

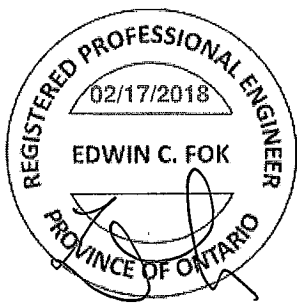
PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	3.0	4.0	1.50	1.00
B	TMWW-t	MT20	3.0	4.0	1.50	1.50
C	TMWW-t	MT20	3.0	4.0	1.50	1.50
D	TTW+m	MT20	3.0	4.0		
E	TMVW-t	MT20	3.0	4.0		
F	BMV1+p	MT20	2.0	4.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BBWW-t	MT20	5.0	6.0	3.00	3.00
I	BBWW-t	MT20	4.0	6.0	2.00	4.50
J	BMV1+p	MT20	2.0	4.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 125.3 lbs FACTORED DOWN AT 4-8-11 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	417	0	417	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
J	501	0	501	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	296	186 / 0	0 / 0	0 / 0	0 / 0	110 / 0	0 / 0
J	358	212 / 0	0 / 0	0 / 0	0 / 0	147 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-204 / 0	-78.0	-78.0	0.03 (1)	6.25	I- B	-269 / 0	0.04 (1)	
B- C	-319 / 0	-78.0	-78.0	0.04 (1)	6.25	B- H	0 / 155	0.04 (1)	
C- D	-148 / 0	-78.0	-78.0	0.04 (1)	6.25	H- C	0 / 86	0.02 (4)	
D- E	-112 / 0	-108.0	-108.0	0.03 (1)	6.25	C- G	-200 / 0	0.04 (1)	
F- E	-398 / 0	0.0	0.0	0.08 (1)	7.81	G- D	-174 / 0	0.04 (1)	
J- A	-262 / 0	0.0	0.0	0.03 (1)	7.81	G- E	0 / 355	0.09 (1)	
						A- I	0 / 194	0.05 (1)	
K- J	0 / 0	-96.5	-96.5	0.17 (1)	10.00				
J- I	-1 / 0	-25.6	-25.6	0.17 (1)	10.00				
I- H	0 / 213	-25.6	-25.6	0.04 (1)	10.00				
H- G	0 / 267	-25.6	-25.6	0.07 (1)	10.00				
G- F	0 / 0	-25.6	-25.6	0.01 (4)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
D	4-8-11	-125	-125	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	21.0	PSF	
	DL =	6.0	PSF	
BOT CH.	LL =	0.0	PSF	
	DL =	7.4	PSF	
TOTAL LOAD	=	34.4	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 4-8-11
RIGHT SETBACK = 0-0
END SETBACK = 3-6-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 656 (0.03")

CSI: TC=0.08/1.00 (E-F:1) , BC=0.17/1.00 (I-J:1) , WB=0.09/1.00 (E-G:1) , SSI=0.13/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

A-18023470

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292590	J1T	3	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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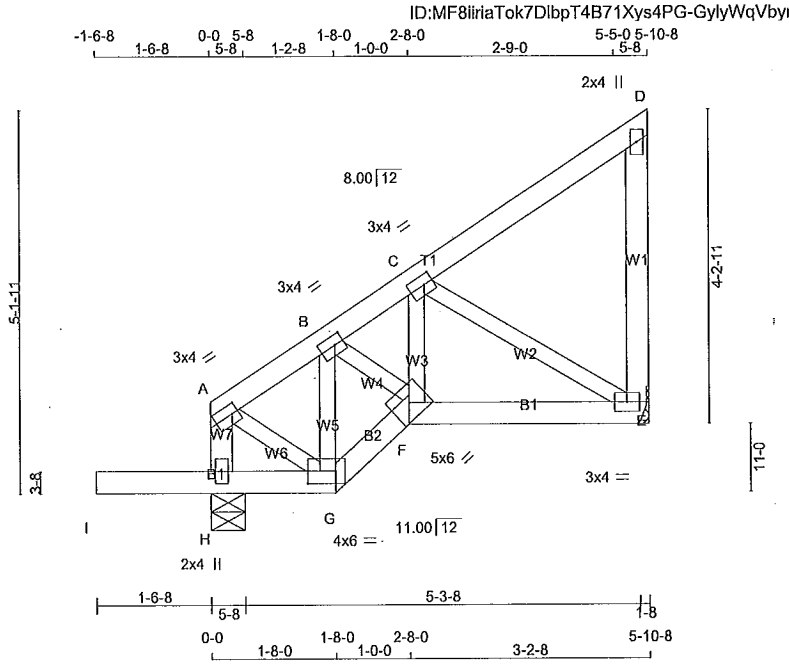

TOTAL WEIGHT = 3 X 30 = 89 lb

(M)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
H - A	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	3.0	4.0	1.50	1.00
B	TMWW-t	MT20	3.0	4.0	1.50	1.50
C	TMWW-t	MT20	3.0	4.0	1.50	1.50
D	TMV+p	MT20	2.0	4.0		
E	BMVW1-t	MT20	3.0	4.0		
F	BBWW-h	MT20	5.0	6.0		
G	BBWW-l	MT20	4.0	6.0	2.00	4.50
H	BMV1+p	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	452	0	452	0	0	5-8	1-8
E	264	0	264	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	322	197 / 0	0 / 0	0 / 0	0 / 0	125 / 0	0 / 0
E	188	115 / 0	0 / 0	0 / 0	0 / 0	73 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- A	-219 / 0	0.0	0.0	0.02 (1)	7.81	A- G	0 / 149	0.03 (1)
A- B	-159 / 0	-78.0	-78.0	0.03 (1)	6.25	G- B	-223 / 0	0.03 (1)
B- C	-250 / 0	-78.0	-78.0	0.09 (1)	6.25	B- F	0 / 154	0.03 (1)
C- D	-12 / 0	-78.0	-78.0	0.09 (1)	6.25	F- C	0 / 52	0.02 (4)
E- D	-100 / 0	0.0	0.0	0.03 (1)	7.81	C- E	-268 / 0	0.06 (1)
I- H	0 / 0	-96.5	-96.5	0.16 (1)	10.00			
H- G	-1 / 0	-18.5	-18.5	0.16 (1)	10.00			
G- F	0 / 162	-18.5	-18.5	0.03 (1)	10.00			
F- E	0 / 231	-18.5	-18.5	0.08 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 626 (0.03")

CSI: TC=0.09/1.00 (B-C:1) , BC=0.16/1.00 (G-H:1) , WB=0.06/1.00 (C-E:1) , SSI=0.12/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.49 (A) (INPUT = 0.90)
JSI METAL= 0.09 (E) (INPUT = 1.00)

A-18023469

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292590	H8T	5	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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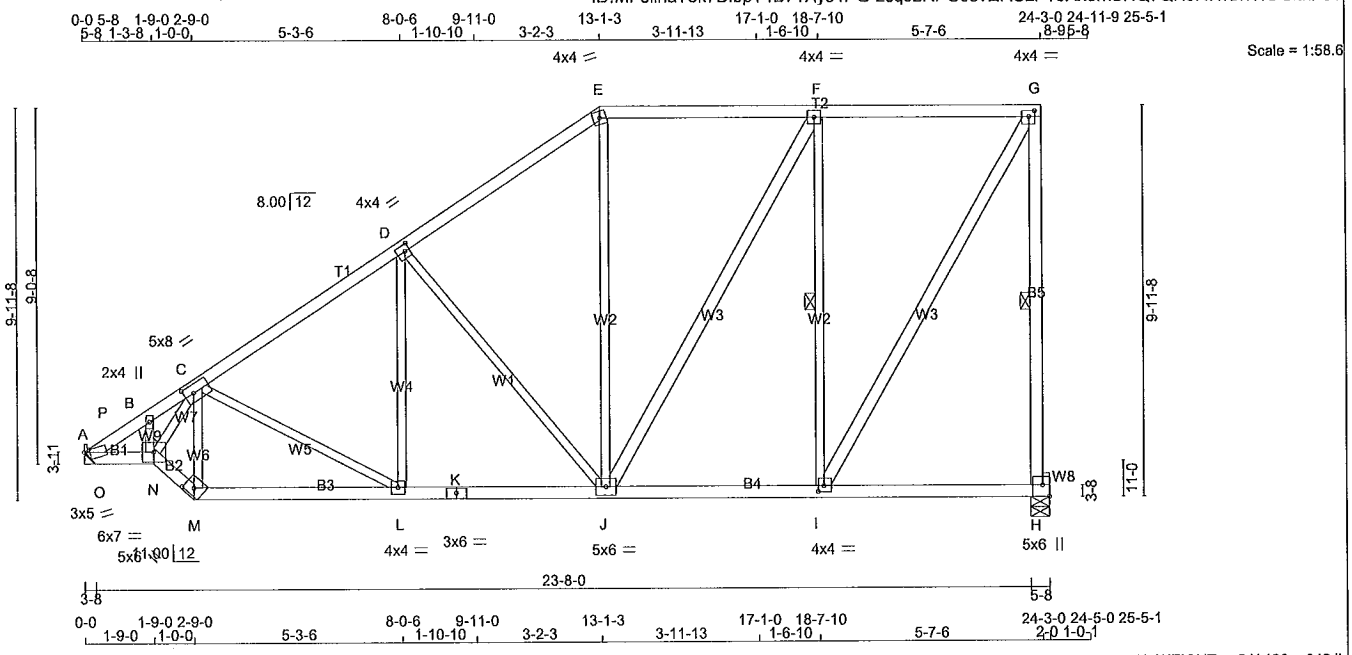


[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
A - N	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
[illegible] PT	2x4	DRY	No.2	SPF
[illegible]	2x4	DRY	No.2	SPF

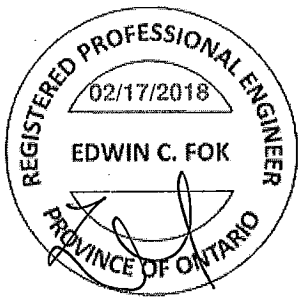
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM1-m	MT20	3.0	5.0	0.50	1.75
B	TMW+w	MT20	2.0	4.0		
C	TMWWW-t	MT20	5.0	8.0	2.50	2.75
D	TMWW-t	MT20	4.0	4.0	2.00	1.50
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW-t	MT20	4.0	4.0	1.75	1.75
H	BMVW1+p	MT20	5.0	6.0	3.50	Edge
I	BMWW-t	MT20	4.0	4.0	1.75	1.75
J	BMWWW-t	MT20	5.0	6.0		
K	BS-t	MT20	3.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BBW-h	MT20	5.0	6.0	2.00	3.00
N	BBWW-t	MT20	6.0	7.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
A	1181	0	1181	0	0	IN-SX	IN-SX
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	
H	1226	0	1226	0	0	5-8	1-13

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	844	506 / 0	0 / 0	0 / 0	0 / 0	338 / 0	0 / 0
H	880	506 / 0	0 / 0	0 / 0	0 / 0	374 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

FOR SECTION E-G, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.68 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-H, F-I.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- P	-1890 / 0	-78.0	-78.0	0.04 (1)	4.88
P- B	-1973 / 0	-78.0	-78.0	0.16 (1)	4.68
B- C	-1854 / 0	-78.0	-78.0	0.17 (1)	4.79
C- D	-1266 / 0	-78.0	-78.0	0.26 (1)	5.45
D- E	-920 / 0	-78.0	-78.0	0.25 (1)	6.14
E- F	-746 / 0	-85.5	-85.5	0.34 (1)	2.00
F- G	-571 / 0	-85.5	-85.5	0.33 (1)	2.00
A- O	0 / 1598	-18.5	-18.5	0.34 (1)	10.00
O- N	0 / 1599	-18.5	-18.5	0.34 (1)	10.00
N- M	0 / 1466	-18.5	-18.5	0.24 (1)	10.00
M- L	0 / 1116	-18.5	-18.5	0.24 (1)	10.00
L- K	0 / 1061	-18.5	-18.5	0.23 (1)	10.00
K- J	0 / 1061	-18.5	-18.5	0.23 (1)	10.00
J- I	0 / 571	-18.5	-18.5	0.19 (4)	10.00
I- H	0 / 0	-18.5	-18.5	0.14 (4)	10.00
H- G	-1186 / 0	0.0	0.0	0.57 (1)	5.87

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- B	0 / 165	0.04 (1)
N- C	0 / 1073	0.24 (1)
M- C	-936 / 0	0.17 (1)
J- E	0 / 162	0.04 (4)
C- L	-62 / 0	0.04 (1)
L- D	0 / 132	0.04 (4)
D- J	-497 / 0	0.69 (1)
J- F	0 / 358	0.06 (1)
I- F	-894 / 0	0.60 (1)
I- G	0 / 1160	0.19 (1)
O- P	-211 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.80")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.80")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.34/1.00 (E-F:1) , BC=0.57/1.00 (G-H:1) , WB=0.69/1.00 (D-J:1) , SSI=0.23/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (A) (INPUT = 0.90)
JSI METAL= 0.54 (A) (INPUT = 1.00)

A-18023468

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292590	H3T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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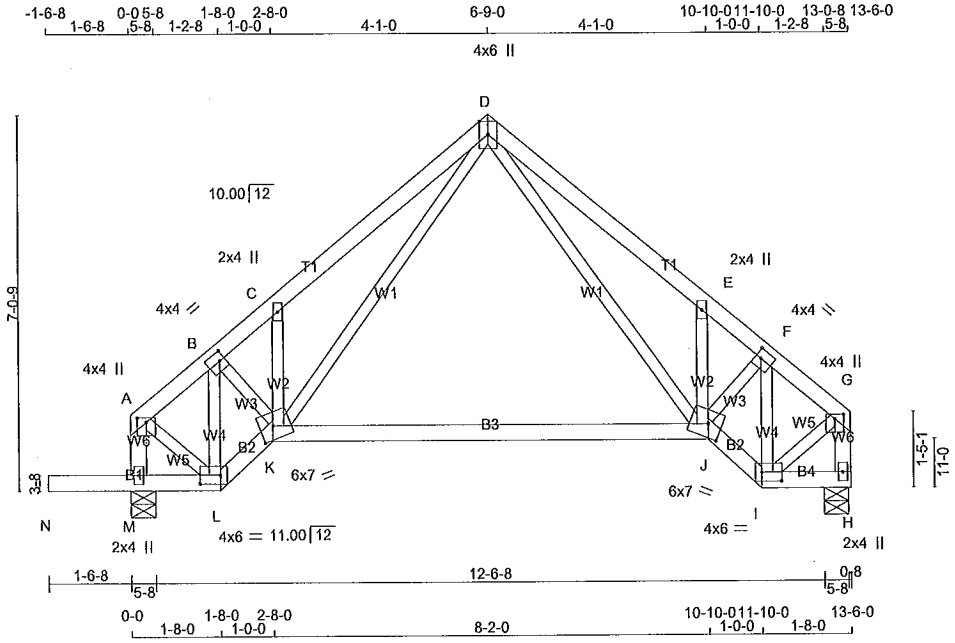


TOTAL WEIGHT = 64 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
M - A	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
EBS	2x3	DRY	No.2	SPF
E. PT				

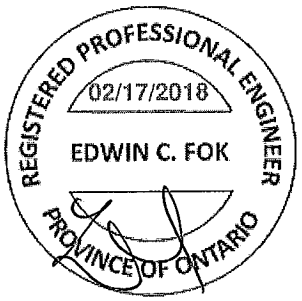
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TMW+w	MT20	2.0	4.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0	2.00	1.25
G	TMVW+p	MT20	4.0	4.0	1.00	2.00
H	BMV1+p	MT20	2.0	4.0		
I	BBWW-I	MT20	4.0	6.0	2.00	4.50
J	BBWWW-m	MT20	6.0	7.0	3.00	3.00
K	BBWWW-m	MT20	6.0	7.0	3.00	3.00
L	BBWW-I	MT20	4.0	6.0	2.00	4.50
M	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	643	0	643	0	0	5-8	1-8
M	808	0	808	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	458	280 / 0	0 / 0	0 / 0	0 / 0	179 / 0	0 / 0
M	576	352 / 0	0 / 0	0 / 0	0 / 0	225 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-454 / 0	-78.0	-78.0	0.03 (1)		6.25
B- C	-707 / 0	-78.0	-78.0	0.11 (1)		6.25
C- D	-791 / 0	-78.0	-78.0	0.16 (1)		6.25
D- E	-829 / 0	-78.0	-78.0	0.16 (1)		6.25
E- F	-745 / 0	-78.0	-78.0	0.11 (1)		6.25
F- G	-500 / 0	-78.0	-78.0	0.04 (1)		6.25
M- A	-575 / 0	0.0	0.0	0.06 (1)		7.81
H- G	-627 / 0	0.0	0.0	0.06 (1)		7.81
N- M	0 / 0	-96.5	-96.5	0.16 (1)		10.00
M- L	-1 / 0	-18.5	-18.5	0.12 (1)		10.00
L- K	0 / 436	-18.5	-18.5	0.07 (1)		10.00
K- J	0 / 344	-18.5	-18.5	0.39 (4)		10.00
J- I	0 / 484	-18.5	-18.5	0.08 (1)		10.00
I- H	0 / 0	-18.5	-18.5	0.02 (4)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D- J	0 / 484	0.11 (1)
J- E	-331 / 0	0.06 (1)
J- F	0 / 437	0.10 (1)
I- F	-545 / 0	0.09 (1)
K- D	0 / 434	0.10 (1)
K- C	-332 / 0	0.06 (1)
B- K	0 / 449	0.10 (1)
L- B	-559 / 0	0.09 (1)
A- L	0 / 410	0.09 (1)
I- G	0 / 452	0.10 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.45")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.45")
CALCULATED VERT. DEFL.(TL) = L/ 709 (0.23")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.16/1.00 (D-E:1) , BC=0.39/1.00 (J-K:4) , WB=0.11/1.00 (D-J:1) , SSI=0.12/1.00 (M-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (K) (INPUT = 0.90)
JSI METAL= 0.18 (E) (INPUT = 1.00)

A-18023466

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292590	H2T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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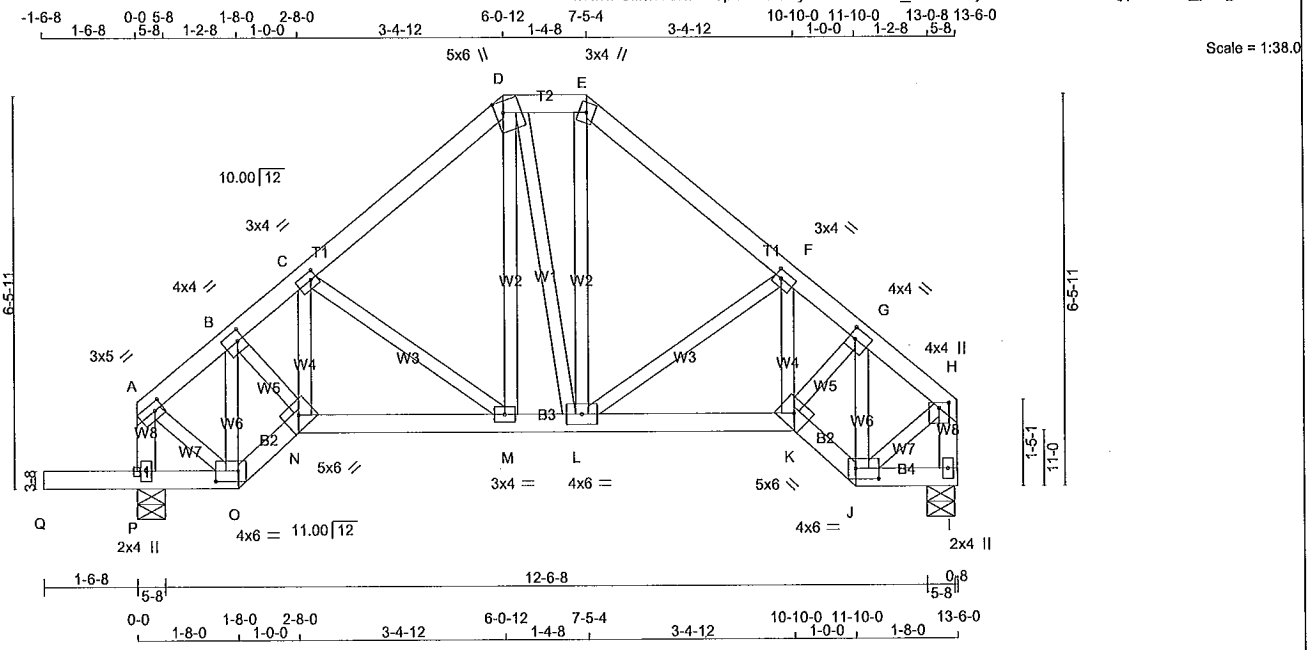


TOTAL WEIGHT = 72 lb

[M][F]

LUMBER

N. L. G. A. RULES

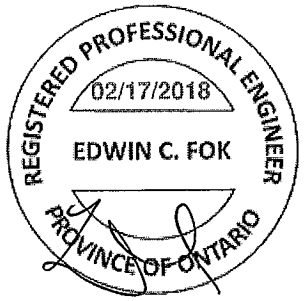
CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
P - A	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
	2x4	DRY	No.2	SPF
AL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	3.0	5.0	1.50	1.75
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TMWW-t	MT20	3.0	4.0	1.50	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TTW+m	MT20	3.0	4.0		
F	TMWW-t	MT20	3.0	4.0	1.50	1.25
G	TMWW-t	MT20	4.0	4.0	2.00	1.25
H	TMVW+p	MT20	4.0	4.0	1.00	2.00
I	BMV1+p	MT20	2.0	4.0		
J	BBWW-l	MT20	4.0	6.0	2.00	4.50
K	BBWW-h	MT20	5.0	6.0		
L	BMWWW-t	MT20	4.0	6.0		
M	BMWW-t	MT20	3.0	4.0		
N	BBWW-h	MT20	5.0	6.0		
O	BBWW-l	MT20	4.0	6.0	2.00	4.50
P	BMV1+p	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	643	0	643	0	0	5-8	1-8
P	808	0	808	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	458	280 / 0	0 / 0	0 / 0	0 / 0	179 / 0	0 / 0
P	576	352 / 0	0 / 0	0 / 0	0 / 0	225 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, P

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-454 / 0	-78.0	-78.0	0.03 (1)	6.25
B- C	-706 / 0	-78.0	-78.0	0.08 (1)	6.25
C- D	-539 / 0	-78.0	-78.0	0.10 (1)	6.25
D- E	-409 / 0	-78.0	-78.0	0.02 (1)	6.25
E- F	-544 / 0	-78.0	-78.0	0.10 (1)	6.25
F- G	-744 / 0	-78.0	-78.0	0.08 (1)	6.25
G- H	-500 / 0	-78.0	-78.0	0.04 (1)	6.25
P- A	-575 / 0	0.0	0.0	0.06 (1)	7.81
I- H	-627 / 0	0.0	0.0	0.06 (1)	7.81
Q- P	0 / 0	-96.5	-96.5	0.16 (1)	10.00
P- O	-1 / 0	-18.5	-18.5	0.12 (1)	10.00
O- N	0 / 436	-18.5	-18.5	0.07 (1)	10.00
N- M	0 / 575	-18.5	-18.5	0.12 (1)	10.00
M- L	0 / 405	-18.5	-18.5	0.10 (1)	10.00
L- K	0 / 603	-18.5	-18.5	0.13 (1)	10.00
K- J	0 / 484	-18.5	-18.5	0.08 (1)	10.00
J- I	0 / 0	-18.5	-18.5	0.02 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- B	-559 / 0	0.09 (1)
B- N	0 / 419	0.09 (1)
N- C	0 / 35	0.01 (4)
C- M	-215 / 0	0.06 (1)
M- D	0 / 161	0.04 (1)
D- L	0 / 17	0.00 (1)
L- E	0 / 181	0.04 (1)
L- F	-244 / 0	0.07 (1)
K- F	0 / 49	0.02 (4)
K- G	0 / 409	0.09 (1)
J- G	-545 / 0	0.09 (1)
A- O	0 / 410	0.09 (1)
J- H	0 / 451	0.10 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.45")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.45")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 829 (0.02")

CSI: TC=0.10/1.00 (E-F:1) , BC=0.16/1.00 (P-Q:1) , WB=0.10/1.00 (H-J:1) , SSI=0.12/1.00 (P-Q:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (A) (INPUT = 0.90)
JSI METAL= 0.15 (A) (INPUT = 1.00)

A-18023465

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292590	H1T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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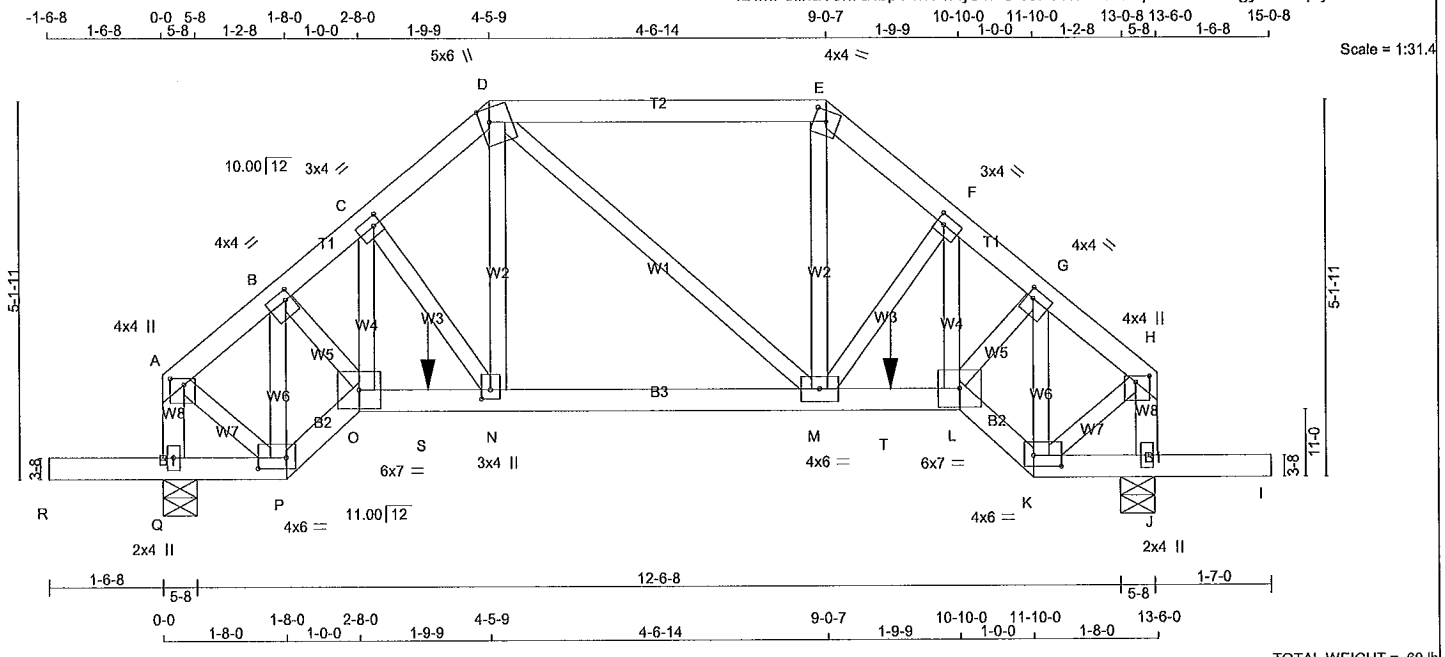


TOTAL WEIGHT = 69 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
Q - A	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
[illegible]	2x4	DRY	No.2	SPF
[illegible]EBS	2x3	DRY	No.2	SPF

EXCEPT

DRY: SEASONED LUMBER.

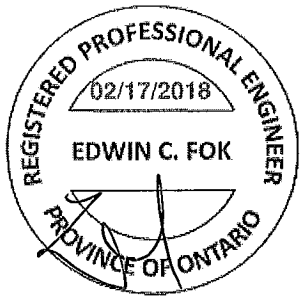
PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.25
B	TMWW-t	MT20	4.0	4.0	1.50	1.00
C	TMWW-t	MT20	3.0	4.0	1.50	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TTW-m	MT20	4.0	4.0	1.75	2.00
F	TMWW-t	MT20	3.0	4.0	1.50	1.25
G	TMWW-t	MT20	4.0	4.0	1.50	1.00
H	TMVW+p	MT20	4.0	4.0	1.00	2.25
J	BMV1+p	MT20	2.0	4.0		
K	BBWW-l	MT20	4.0	6.0	1.75	4.50
L	BBWW-l	MT20	6.0	7.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BMWW+t	MT20	3.0	4.0	1.50	1.50
O	BBWW-l	MT20	6.0	7.0		
P	BBWW-l	MT20	4.0	6.0	1.75	4.50
Q	BMV1+p	MT20	2.0	4.0		

HANGERS NOTES

1)

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
Q	1520	0	1520	0	0	5-8	2-4
J	1520	0	1520	0	0	5-8	2-4

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1076	700 / 0	0 / 0	0 / 0	0 / 0	376 / 0	0 / 0
J	1076	700 / 0	0 / 0	0 / 0	0 / 0	376 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.96 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1084 / 0	-78.0	-78.0	0.06 (1)	5.98
B- C	-1751 / 0	-78.0	-78.0	0.08 (1)	4.96
C- D	-1589 / 0	-78.0	-78.0	0.08 (1)	5.15
D- E	-1239 / 0	-78.0	-78.0	0.34 (1)	5.30
E- F	-1592 / 0	-78.0	-78.0	0.08 (1)	5.14
F- G	-1751 / 0	-78.0	-78.0	0.08 (1)	4.96
G- H	-1084 / 0	-78.0	-78.0	0.06 (1)	5.98
Q- A	-1287 / 0	0.0	0.0	0.15 (1)	7.07
J- H	-1287 / 0	0.0	0.0	0.15 (1)	7.07
R- Q	0 / 0	-96.5	-96.5	0.17 (1)	10.00
Q- P	-1 / 0	-18.5	-18.5	0.17 (1)	10.00
P- O	0 / 1048	-18.5	-18.5	0.19 (1)	10.00
O- S	0 / 1337	-18.5	-18.5	0.49 (1)	10.00
S- N	0 / 1337	-112.0	-112.0	0.49 (1)	10.00
N- M	0 / 1237	-112.0	-112.0	0.48 (1)	10.00
M- T	0 / 1337	-112.0	-112.0	0.50 (1)	10.00
T- L	0 / 1337	-18.5	-18.5	0.50 (1)	10.00
L- K	0 / 1048	-18.5	-18.5	0.19 (1)	10.00
K- J	-1 / 0	-18.5	-18.5	0.17 (1)	10.00
J- I	0 / 0	-96.5	-96.5	0.17 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- B	-1281 / 0	0.23 (1)
B- O	0 / 943	0.23 (1)
O- C	0 / 155	0.04 (1)
C- N	-208 / 0	0.04 (1)
N- D	0 / 776	0.19 (1)
D- M	0 / 3	0.00 (1)
M- E	0 / 780	0.19 (1)
M- F	-205 / 0	0.04 (1)
L- F	0 / 150	0.04 (1)
L- G	0 / 943	0.23 (1)
K- G	-1281 / 0	0.23 (1)
A- P	0 / 983	0.24 (1)
K- H	0 / 983	0.24 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
S	3-7-4	-426	-426	---	FRONT	VERT	TOTAL
T	9-10-12	-426	-426	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CStdGirder
START DISTANCE = 3-7-4
START SPAN CARRIED = 5-10-8
END DISTANCE = 9-10-12
END SPAN CARRIED = 5-10-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.45")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.45")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.34/1.00 (D-E:1) , BC=0.50/1.00 (L-M:1) , WB=0.24/1.00 (H-K:1) , SSI=0.33/1.00 (L-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

A-18023464

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292589	H3	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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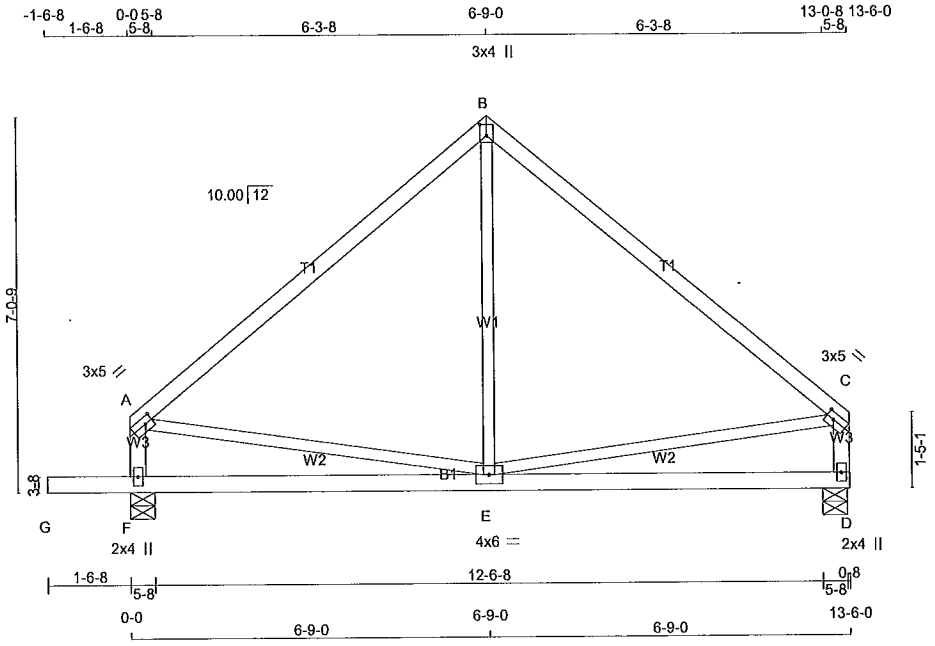


TOTAL WEIGHT = 56 lb

[M][F]

LUMBER

N. L. G. A. RULES

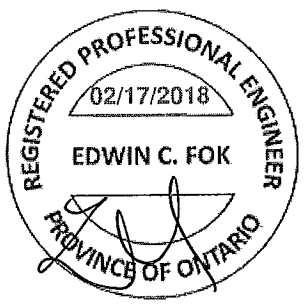
CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	2100F 1.8E	SPF
B - C	2x4	DRY	2100F 1.8E	SPF
F - A	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
G - D	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	3.0	5.0	1.50	1.75
B	TTW+p	MT20	3.0	4.0	2.50	1.50
C	TMVW-t	MT20	3.0	5.0	1.50	1.75
D	BMV1+p	MT20	2.0	4.0		
E	BMWWW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	651	0	651	0	0	5-8	1-8
F	800	0	800	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	464	283 / 0	0 / 0	0 / 0	0 / 0	181 / 0	0 / 0
F	570	348 / 0	0 / 0	0 / 0	0 / 0	222 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-446 / 0	-78.0	-78.0	0.30 (1)	6.25	E- B	0 / 128	0.04 (4)
B- C	-446 / 0	-78.0	-78.0	0.30 (1)	6.25	A- E	0 / 347	0.08 (1)
F- A	-604 / 0	0.0	0.0	0.06 (1)	7.81	E- C	0 / 347	0.08 (1)
D- C	-604 / 0	0.0	0.0	0.06 (1)	7.81			
G- F	0 / 0	-96.5	-96.5	0.16 (1)	10.00			
F- E	0 / 0	-18.5	-18.5	0.24 (4)	10.00			
E- D	0 / 0	-18.5	-18.5	0.24 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.45")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.45")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.30/1.00 (A-B:1) , BC=0.24/1.00 (E-F:4) , WB=0.08/1.00 (C-E:1) , SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.54 (A) (INPUT = 0.90)
JSI METAL= 0.18 (C) (INPUT = 1.00)

A-18023433

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292589	H2	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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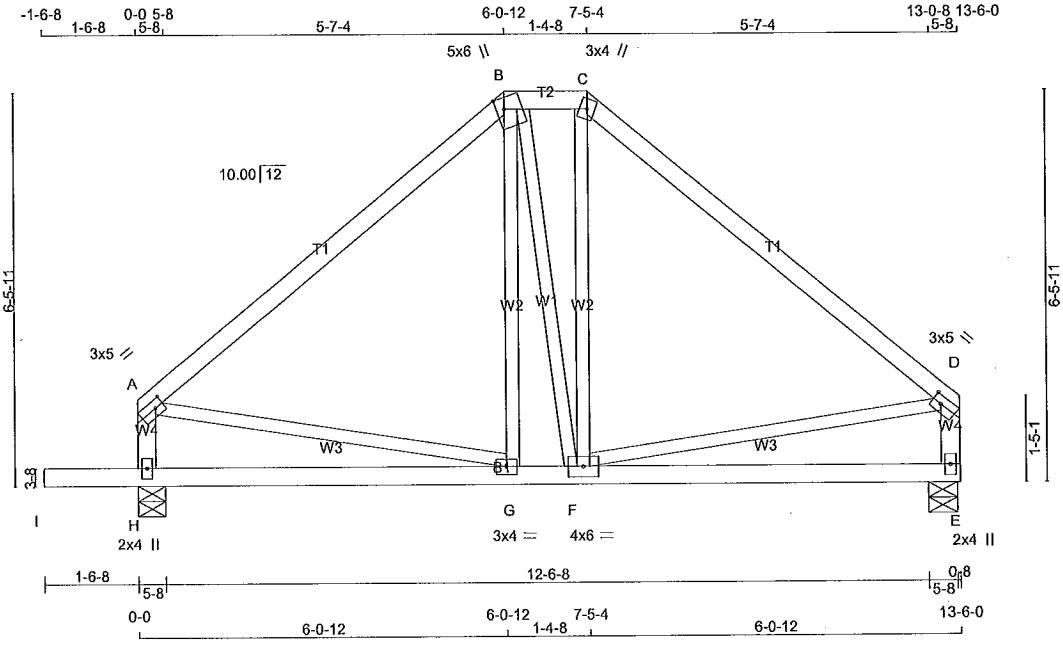


TOTAL WEIGHT = 64 lb

[M][F]

LUMBER

N. L. G. A. RULES

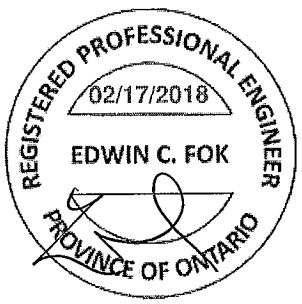
CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
I - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

EASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	3.0	5.0	1.50	1.75
B	TTWW+m	MT20	5.0	6.0	2.25	1.50
C	TTW+m	MT20	3.0	4.0		
D	TMVW-t	MT20	3.0	5.0	1.50	1.75
E	BMV1+p	MT20	2.0	4.0		
F	BMWWW-t	MT20	4.0	6.0		
G	BMWW-t	MT20	3.0	4.0		
H	BMV1+p	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	651	0	651	0	0	5-8	1-8
H	800	0	800	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	464	283 / 0	0 / 0	0 / 0	0 / 0	181 / 0	0 / 0
H	570	348 / 0	0 / 0	0 / 0	0 / 0	222 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-474 / 0	-78.0	-78.0	0.37 (1)	6.25	G- B	0 / 55	0.02 (4)
B- C	-365 / 0	-78.0	-78.0	0.02 (1)	6.25	B- F	0 / 7	0.00 (4)
C- D	-475 / 0	-78.0	-78.0	0.37 (1)	6.25	F- C	0 / 62	0.02 (4)
H- A	-605 / 0	0.0	0.0	0.06 (1)	7.81	A- G	0 / 369	0.08 (1)
E- D	-604 / 0	0.0	0.0	0.06 (1)	7.81	F- D	0 / 370	0.08 (1)
I- H	0 / 0	-96.5	-96.5	0.16 (1)	10.00			
H- G	0 / 0	-18.5	-18.5	0.16 (4)	10.00			
G- F	0 / 364	-18.5	-18.5	0.21 (4)	10.00			
F- E	0 / 0	-18.5	-18.5	0.17 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.45")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.45")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.37/1.00 (C-D:1) , BC=0.21/1.00 (F-G:4) , WB=0.08/1.00 (D-F:1) , SSI=0.14/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.65 (G) (INPUT = 0.90)
JSI METAL= 0.19 (D) (INPUT = 1.00)

A-18023432

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292589	H1	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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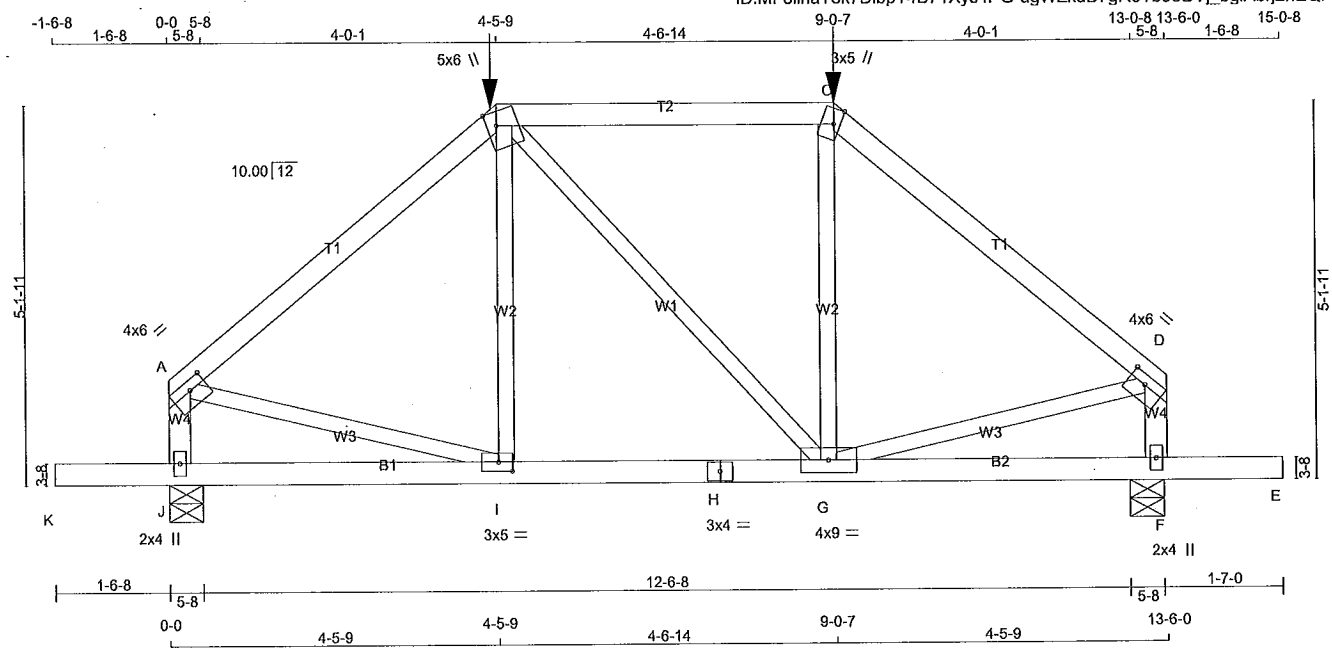


TOTAL WEIGHT = 60 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
J - A	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

[illegible]

SEASONED LUMBER.

PLATES (table is in inches)

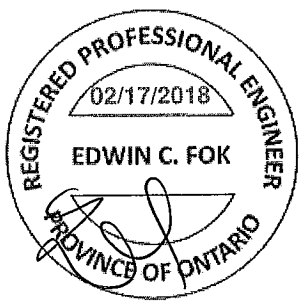
JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0	1.50	2.75
B	TTWW+m	MT20	5.0	6.0	2.25	1.50
C	TTW+m	MT20	3.0	5.0	Edge	1.00
D	TMVW-t	MT20	4.0	6.0	1.50	2.75
F	BMV1+p	MT20	2.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BS-t	MT20	3.0	4.0		
I	BMWW-t	MT20	3.0	5.0	1.50	2.25
J	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 253.8 lbs FACTORED DOWN AT 9-0-7, AND 253.8 lbs FACTORED DOWN AT 4-5-9 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1347	0	1347	0	0	5-8	2-0
F	1347	0	1347	0	0	5-8	2-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	962	578 / 0	0 / 0	0 / 0	0 / 0	385 / 0	0 / 0
F	962	578 / 0	0 / 0	0 / 0	0 / 0	385 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.35 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-1159 / 0	-78.0	-78.0	0.34 (1)	5.47	I- B	-34 / 118	0.04 (4)
B- C	-887 / 0	-153.5	-153.5	0.63 (1)	5.35	B- G	0 / 0	0.00 (1)
C- D	-1159 / 0	-78.0	-78.0	0.34 (1)	5.47	G- C	-34 / 118	0.05 (4)
J- A	-1130 / 0	0.0	0.0	0.13 (1)	7.43	A- I	0 / 913	0.23 (1)
F- D	-1130 / 0	0.0	0.0	0.13 (1)	7.43	G- D	0 / 913	0.23 (1)
K- J	0 / 0	-96.5	-96.5	0.17 (1)	10.00			
J- I	0 / 0	-36.4	-36.4	0.18 (4)	10.00			
I- H	0 / 888	-36.4	-36.4	0.28 (4)	10.00			
H- G	0 / 888	-36.4	-36.4	0.28 (4)	10.00			
G- F	0 / 0	-36.4	-36.4	0.18 (4)	10.00			
F- E	0 / 0	-96.5	-96.5	0.17 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
B	4-5-9	-254	-254	---	FRONT	VERT	TOTAL
C	9-0-7	-254	-254	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 4-5-9
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.45")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.45")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.63/1.00 (B-C:1) , BC=0.28/1.00 (G-I:4) , WB=0.23/1.00 (A-I:1) , SSI=0.30/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

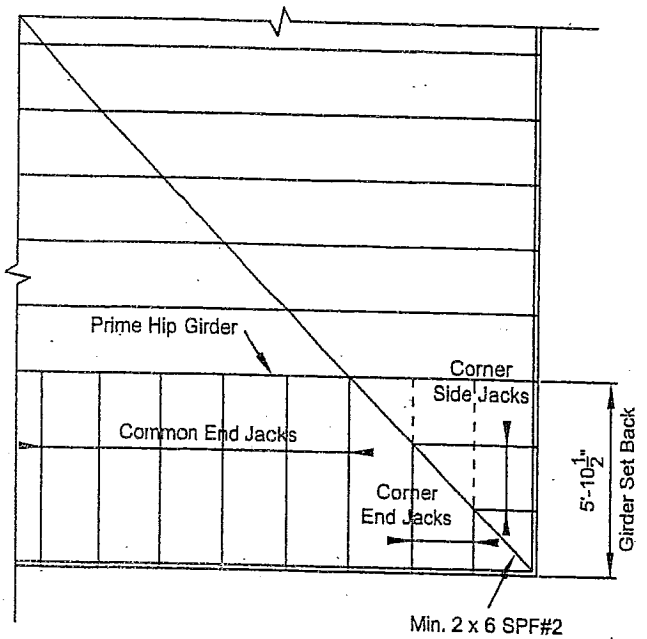
PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

A-18023431

CONTINUED ON PAGE 2

STRACON ENGINEERING INC.



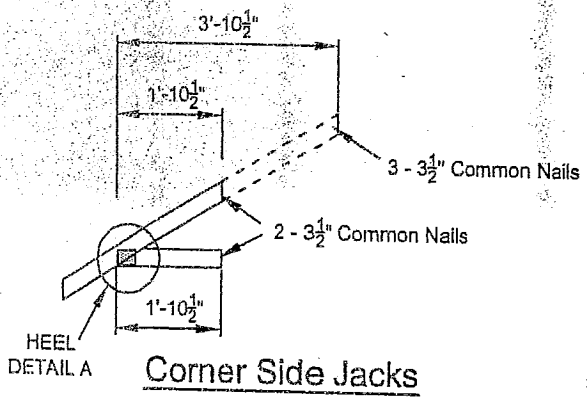
45° Hip End

LUMBER SPECIFICATION

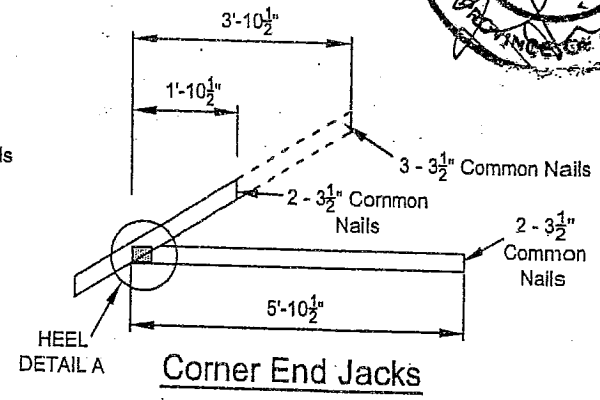
TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

DESIGN LOAD

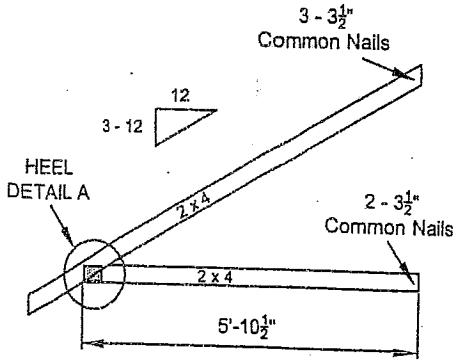
TOP CHORD SNOW LOAD	:	40.5 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD	:	50.5 P.S.F



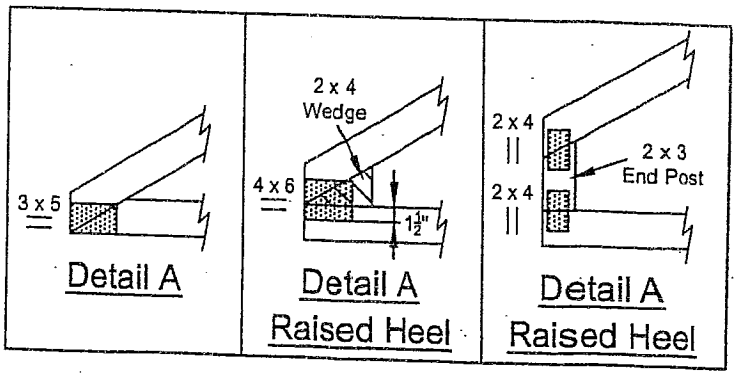
Corner Side Jacks



Corner End Jacks



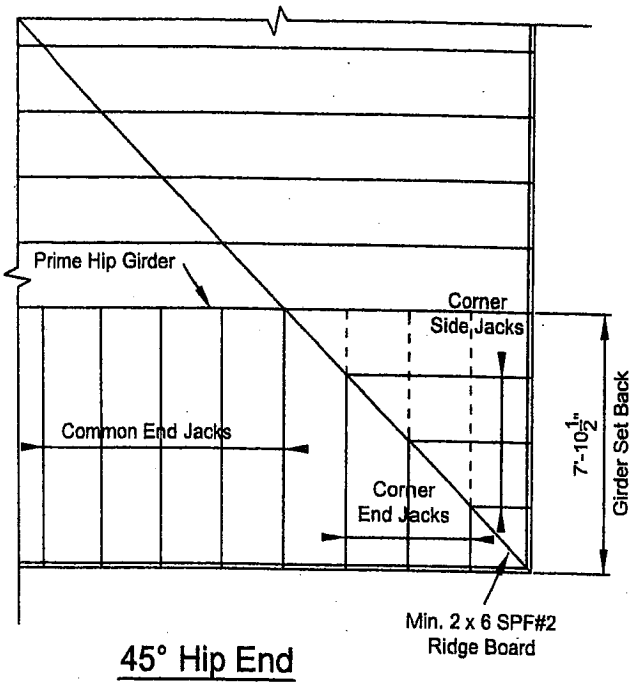
Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

CS-51008

STRACON ENGINEERING INC.



45° Hip End

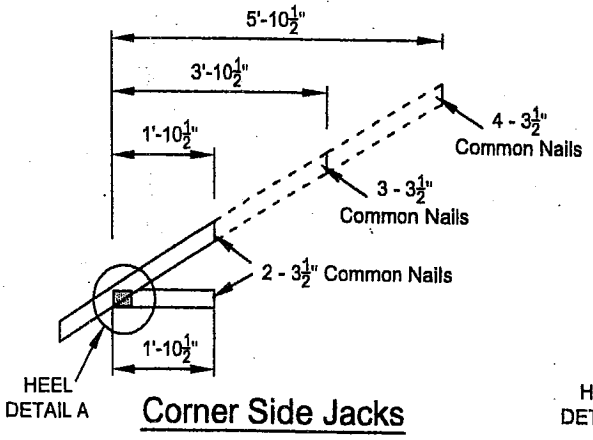
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

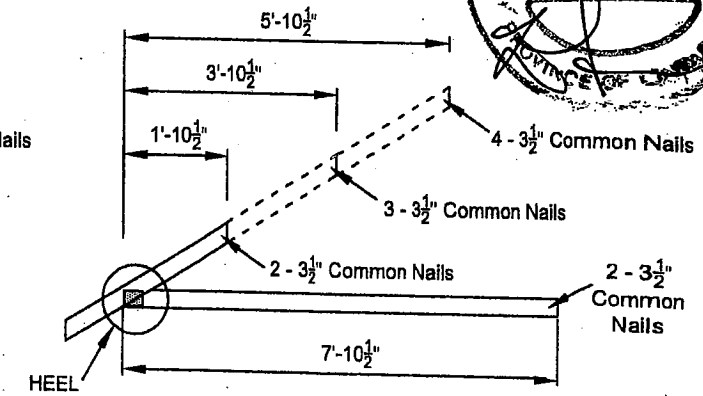
DESIGN LOAD

TOP CHORD SNOW LOAD	:	34.8 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD	:	44.8 P.S.F

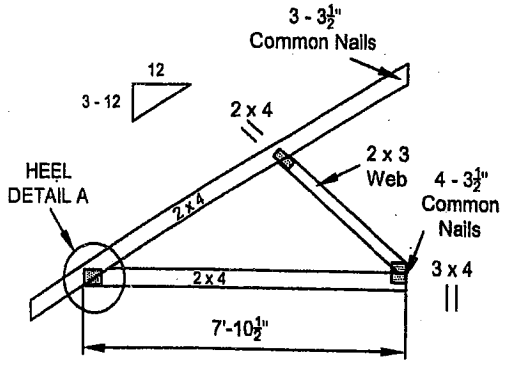
REGISTERED PROFESSIONAL ENGINEER
E. FOK
PROVINCE OF ONTARIO



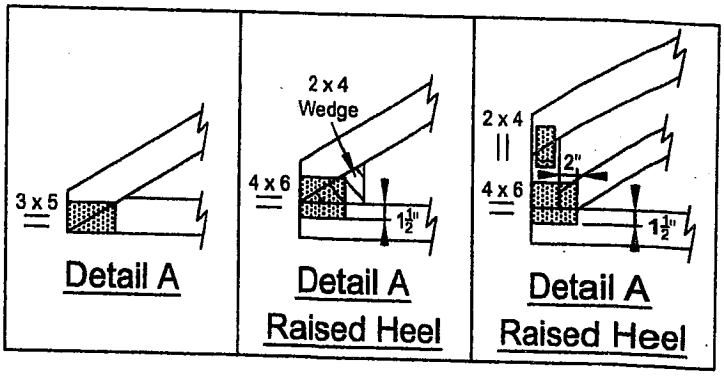
Corner Side Jacks



Corner End Jacks



Common End Jacks



Detail A

Detail A Raised Heel

Detail A Raised Heel

NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

CS-71008N

LUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

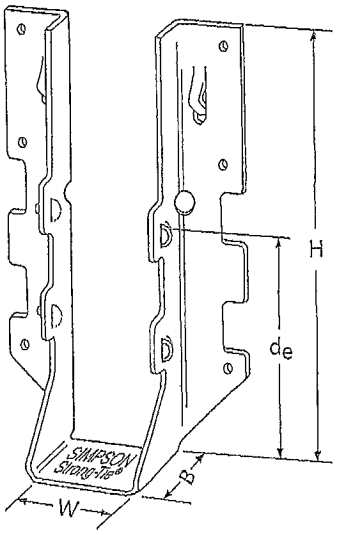
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

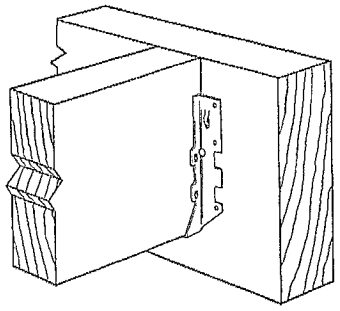
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified

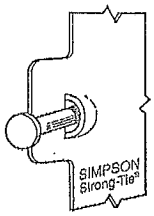


LUS28


Typical LUS Installation

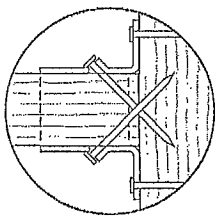
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LUS24	18	1⁹⁄₁₆	3⅛	1¾	1 15⁄16	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3⅛	3⅛	2	1 15⁄16	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1⁹⁄₁₆	4¾	1¾	3⅝	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3⅛	4⅞	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4⅝	4⁹⁄₁₆	2	3¼	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1⁹⁄₁₆	6⅝	1¾	3¾	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3⅛	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4⅝	6¼	2	3¼	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1⁹⁄₁₆	7 13⁄16	1¾	3⅞	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3⅛	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4⅝	8⅜	2	5¼	(8) 16d	(6) 16d	2580	3345	2320	2375

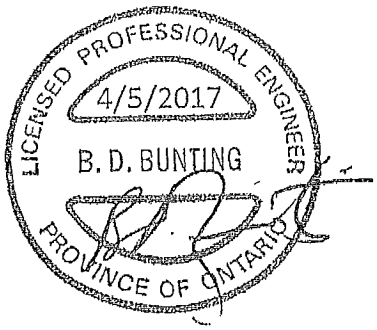
1. d_e is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580


Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2019, and reflects information available as of March 1, 2017. This information is updated periodically and should not be relied upon after June 30, 2019. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

(800) 999-5099
strongtie.com

HUS/LJS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

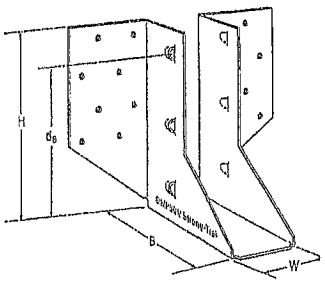
- Factored resistances are in accordance with CSA O86 -14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

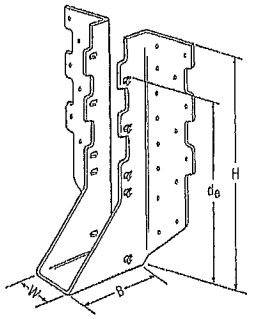
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

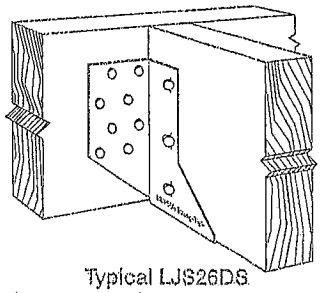
- See current catalogue for options



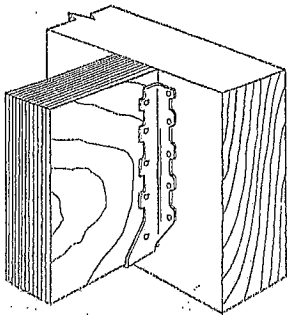
LJS26DS



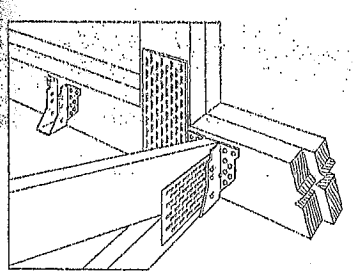
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS Installation



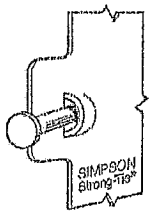
Typical HUS Installation



Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

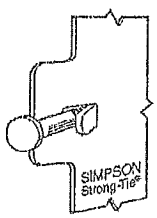
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
								lb.	lb.	lb.	lb.
LJS26DS	18	1 9/16	5	3½	4⅝	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1⅝	5 3/8	3	3 15/16	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1⅝	7 3/32	3	6 3/32	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1⅝	9 3/32	3	7 31/32	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1 13/16	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

1. d_e is the distance from the seat of the hanger to the highest joist nail.

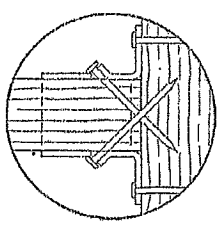


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

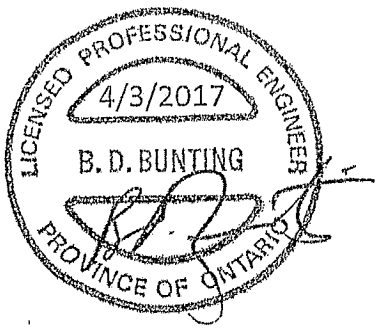
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



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(800) 999-5099
strongtie.com