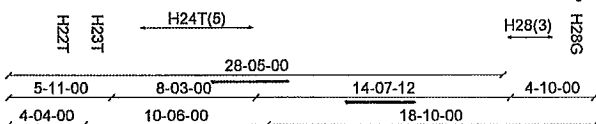
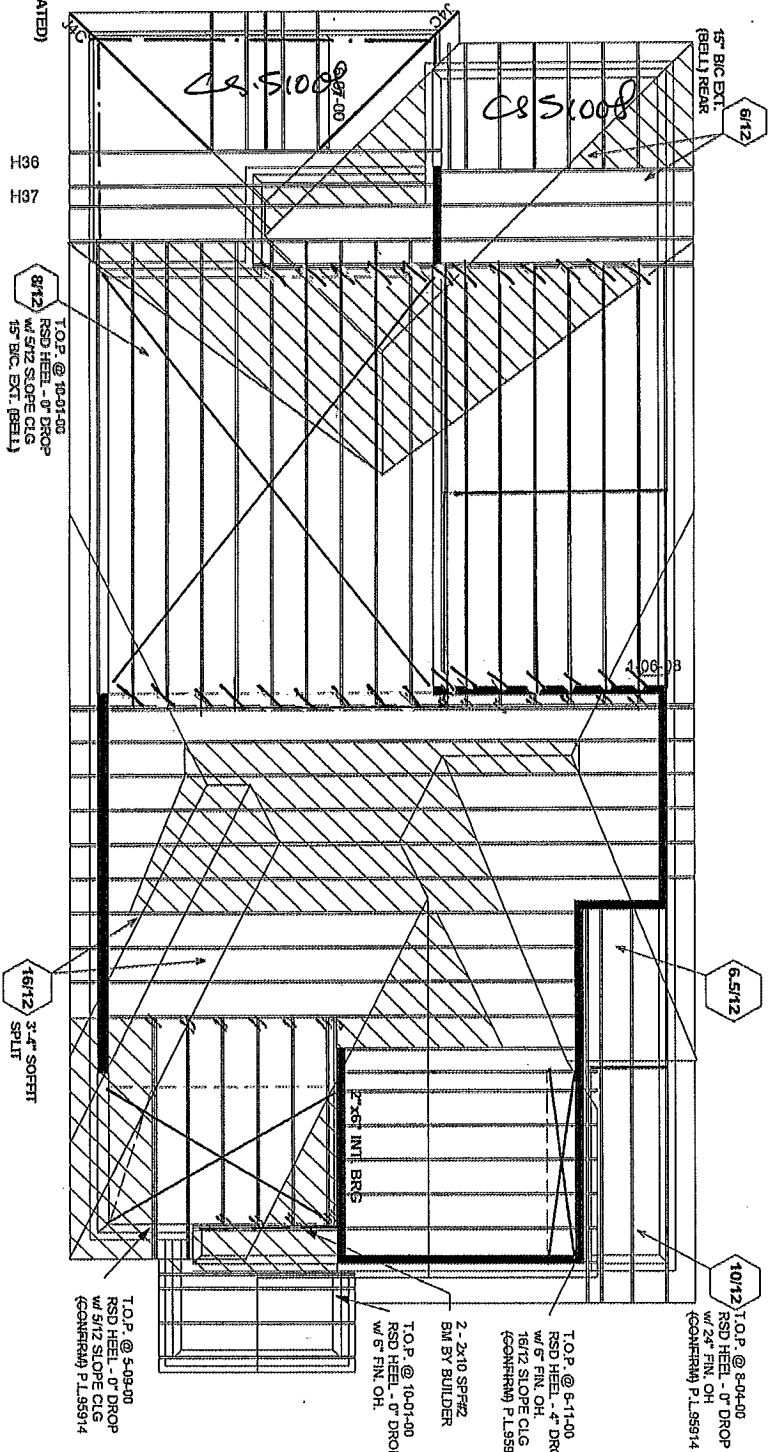
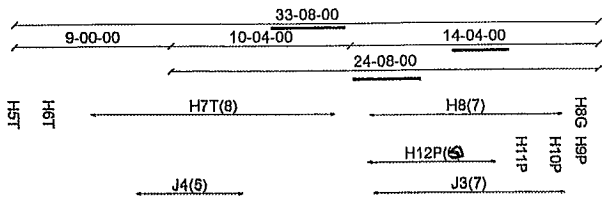


ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. (Latest Edition) ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2X4 S.F. #2, 24" O.C. WITH A VERT. POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DIST. BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 8'



A-18023435 - A-18023463
A-18023471 - A-18023476



DEERBOURNE

Builder / Location
GOLD PARK HOMES / VAUGHAN

Model / Elevator
4201 REV1 / A LOGGIA



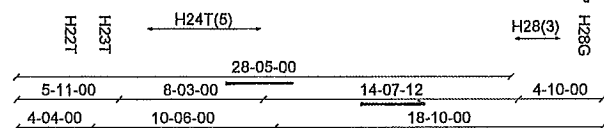
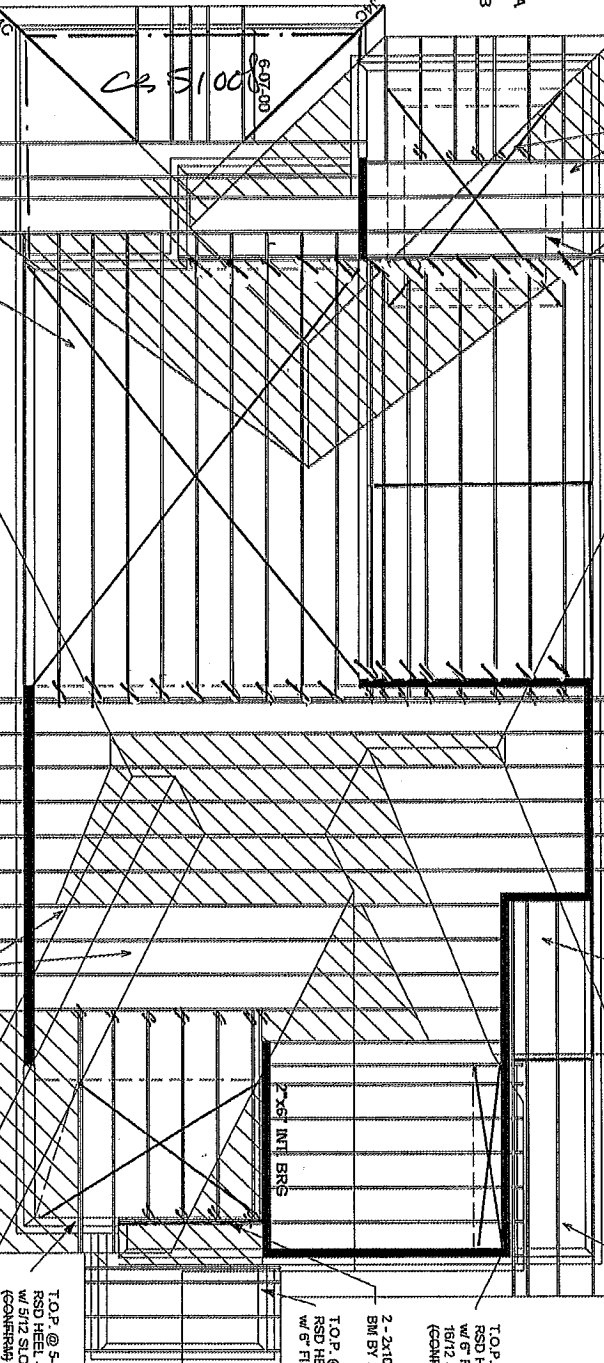
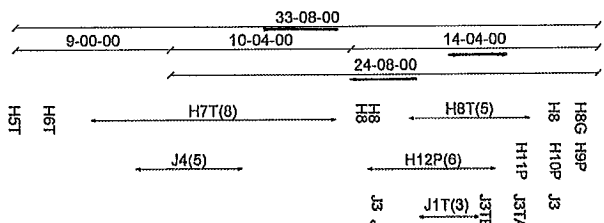
Job Track: 45147
Layout ID: 292591
Plan Log: 95914

Project: PINE VALLEY
Date: 1/3/2018
Designer: AMANDA

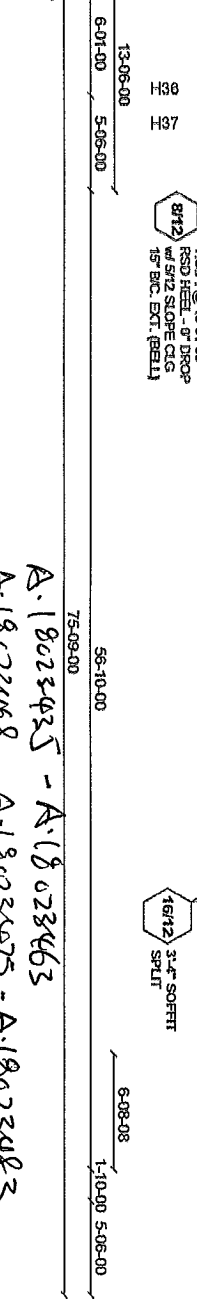
THESE DRAWINGS CONSTITUTE THE PROPERTY OF ALPA ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY ALPA ROOF TRUSSES INC AND WILL BE RETRACTED BY ALPA ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

ALPA-BARRIE

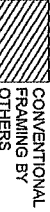
ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. (Latest Edition) ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2X4 SPF #2, 24" O.C. WITH A VERT. POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DIST. BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 8'



15' FIN. OH. (BIC EXT. INDICATED) RSD. HEEL - 0" DROP (2" x 6" FASCIA) - FM ASPHALT SHINGLES 2" x 6" BRICKWORK PIGGYBACK TRS PURLINS BY OTHERS LOFT WALLS SUPPORTED BY OTHERS



A-18023435 - A-18023463
A-18023468, A-18023475 - A-18023483



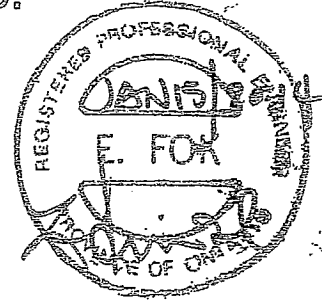
Job Track: 45147
Layout ID: 292592
Plan Log: 95914

Builder / Location: GOLD PARK HOMES / VAUGHAN
Project: PINE VALLEY
Date: 1/3/2018
Designer: AMANDA

Model / Elevation: 4201 REV1 / A LOGGIA W/TRAY
THESE DRAWINGS CONSTITUTE THE PROPERTY OF ALPA ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY ALPA ROOF TRUSSES INC AND WILL BE RETRACTED BY ALPA ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

Stracon Engineering Inc.

69 Graydon Crescent
Richmond Hill, Ontario
L4B 3W7
(905) 832-2250 Fax (905) 832-0286



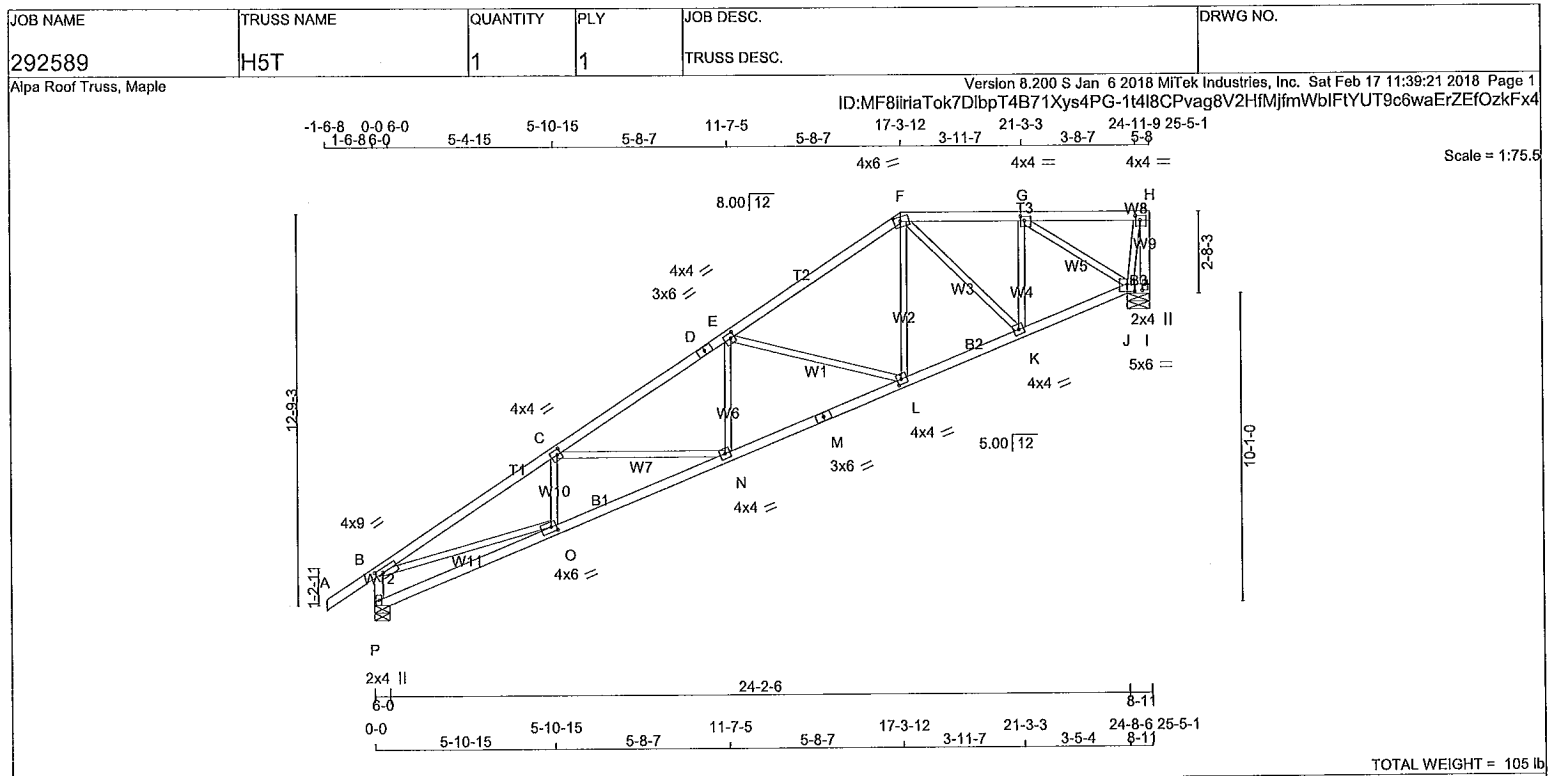
RESPONSIBILITIES

1. Stracon Engineering Inc. is responsible for the design of trusses as individual components.
2. It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities with jurisdictions.
3. All dimensions are to be verified by owner contractor, architect or other authority before manufacture.
4. Stracon Engineering Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Stracon Engineering Inc. drawings is specified for the truss as a component only and forms an integral part of the truss design.
5. It is the truss manufacture's responsibility to ensure that trusses are manufactured in conformance with Stracon Engineering Inc. specifications outlined below.

SPECIFICATIONS

1. Trusses designed by Stracon Engineering Inc. conforms to the relevant section of the Ontario Building Code of Canada (Part 9 or Part 4) or to the Canadian code for farm buildings, whichever applies to the building type indicated on the drawings as well as to the procedures established by the truss plate institute of Canada. Unit stresses used are as per CSA-086-09.
2. Lumber is to be the sizes and grade specified.
3. Moisture content of lumber is not to exceed 19% in service unless otherwise specified.
4. Lumber not to be treated with chemicals unless otherwise specified.
5. Plates shall be applied to both faces of the truss at each joint and shall be positioned as specified.
6. The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals not exceeding 12.5 times in thickness.
7. Where not rigid ceiling is attached directly to the bottom chord, laterally brace the chords at intervals not exceeding 3M (10') o.c.

January 15, 2014



LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER		
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - H	2x4	DRY	No.2	SPF	
I - H	2x4	DRY	No.2	SPF	
P - B	2x4	DRY	No.2	SPF	
P - M	2x4	DRY	No.2	SPF	
M - J	2x4	DRY	No.2	SPF	
J - I	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	9.0	1.75	3.00
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TMVW-t	MT20	4.0	4.0	2.00	1.50
F	TTWW-m	MT20	4.0	6.0	1.75	2.50
G	TMVW-t	MT20	4.0	4.0	1.75	1.50
H	TMVW-t	MT20	4.0	4.0	1.75	1.75
I	BMV1+p	MT20	2.0	4.0	2.25	1.00
J	BBVW1-l	MT20	5.0	6.0	2.75	3.00
K	BMVW-t	MT20	4.0	4.0		
L	BMVW-t	MT20	4.0	4.0	2.00	1.75
M	BS-t	MT20	3.0	6.0		
N	BMVW-t	MT20	4.0	4.0		
O	BMVW-t	MT20	4.0	6.0	2.00	2.00
P	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
I	-986	0	0	0	-986	8-11	7-7	
J	2277	0	2277	0	0	8-11	7-7	
P	1288	0	1288	0	0	6-0	1-15	

PROVIDE ANCHORAGE AT BEARING JOINT I FOR 986 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BLDG. DESIGNER

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM.LIVE				
I	-703	0/-428	0/0	0/0	0/0	0/-275	0/0
J	1624	990/0	0/0	0/0	0/0	634/0	0/0
P	916	574/0	0/0	0/0	0/0	342/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, J, P

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

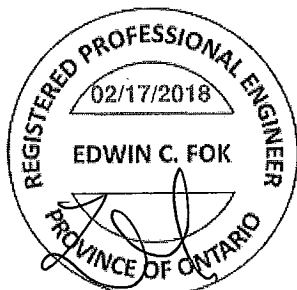
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO		FROM TO			FR-TO		
A-B	0/35	-78.0 -78.0	0.15 (1)	10.00	O-C	-275/0	0.05 (1)
B-C	-2530/0	-78.0 -78.0	0.58 (1)	3.89	C-N	-299/0	0.17 (1)
C-D	-2182/0	-78.0 -78.0	0.52 (1)	4.16	N-E	0/243	0.06 (4)
D-E	-2182/0	-78.0 -78.0	0.52 (1)	4.16	E-L	-811/0	0.52 (1)
E-F	-1268/0	-78.0 -78.0	0.41 (1)	5.24	L-F	0/629	0.14 (1)
F-G	-803/0	-78.0 -78.0	0.23 (1)	6.25	F-K	-337/0	0.16 (1)
G-H	0/339	-78.0 -78.0	0.27 (1)	10.00	K-G	0/401	0.09 (1)
H-I	0/993	0.0 0.0	0.16 (1)	10.00	G-J	-1390/0	0.42 (1)
I-H	0/993	0.0 0.0	0.16 (1)	10.00	J-H	-1167/0	0.19 (1)
P-B	-1238/0	0.0 0.0	0.13 (1)	7.22	B-O	0/2180	0.49 (1)
P-O	0/4	-18.5 -18.5	0.14 (4)	10.00			
O-N	0/2292	-18.5 -18.5	0.43 (1)	10.00			
N-M	0/1968	-18.5 -18.5	0.37 (1)	10.00			
M-L	0/1968	-18.5 -18.5	0.37 (1)	10.00			
L-K	0/1131	-18.5 -18.5	0.23 (1)	10.00			
K-J	0/868	-18.5 -18.5	0.17 (1)	10.00			
J-I	0/0	-18.5 -18.5	0.00 (4)	10.00			

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	34.4	PSF	

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.82")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.82")
CALCULATED VERT. DEFL.(TL) = L/999 (0.22")

CSI: TC=0.58/1.00 (B-C:1), BC=0.43/1.00 (N-O:1)
WB=0.52/1.00 (E-L:1), SSI=0.18/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.65 (M) (INPUT = 1.00)

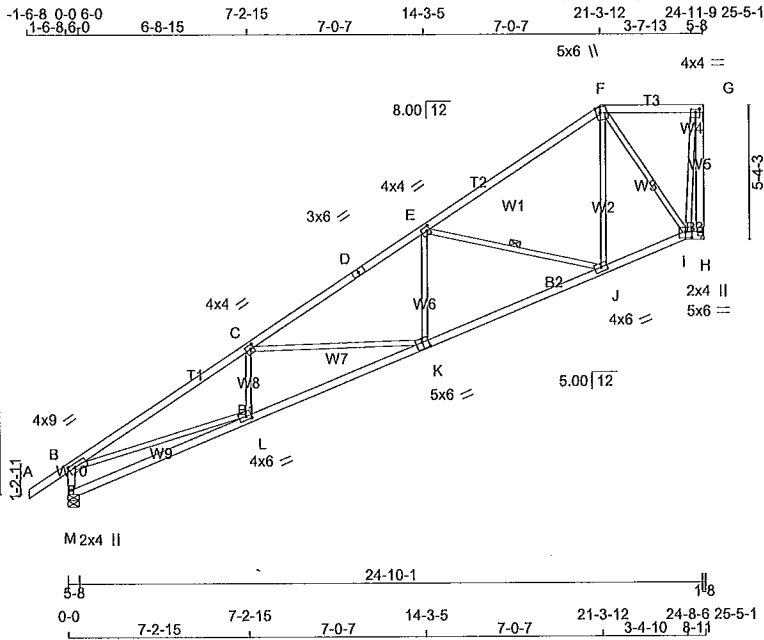
A-18023435

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292589	H6T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Sat Feb 17 11:39:30 2018 Page 1

ID:MF8ilriaTok7DlbpT4B71Xys4PG-Gc791HWZSRHEegr5k2RdSB7MU7WqD9suJIECSMzkFwx



Scale = 1:91.9

TOTAL WEIGHT = 111 lb

(M/F)

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - G	2x4	DRY	No.2	SPF	
H - G	2x4	DRY	No.2	SPF	
M - B	2x4	DRY	No.2	SPF	
M - K	2x4	DRY	No.2	SPF	
K - I	2x4	DRY	No.2	SPF	
I - H	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)							
JT	TYPE	PLATES	W	LEN	Y	X	
B	TMVW-t	MT20	4.0	9.0	1.75	3.00	
C	TMVW-t	MT20	4.0	4.0	2.00	1.50	
D	TS-t	MT20	3.0	6.0			
E	TMVW-t	MT20	4.0	4.0	2.00	1.50	
F	TTVW+m	MT20	5.0	6.0	Edge	1.75	
G	TMVW-t	MT20	4.0	4.0	1.50	1.75	
H	BMV1+p	MT20	2.0	4.0			
I	BBVW-t	MT20	5.0	6.0	2.75	3.00	
J	BMVW-t	MT20	4.0	6.0			
K	BSVW-t	MT20	5.0	6.0	3.00	3.00	
L	BMVW-t	MT20	4.0	6.0	1.75	2.50	
M	BMV1+p	MT20	2.0	4.0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
H	1226	0	1226	0
M	1353	0	1353	0

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND
H	874	534 / 0	0 / 0	0 / 0	341 / 0
M	962	602 / 0	0 / 0	0 / 0	360 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.50 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEB S				
MAX. FACTORED		FACTORED		MAX. UNBRAC	MAX. FACTORED		MAX CSI (LC)		
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1		MEMB.	FORCE (LBS)			
FR-TO		FROM	TO	LENGTH	FR-TO				
A-B	0 / 35	-78.0	-78.0	0.15 (1)	10.00	L-C	-195 / 43	0.04 (1)	
B-C	-2790 / 0	-78.0	-78.0	0.82 (1)	3.50	C-K	-614 / 0	0.61 (1)	
C-D	-2073 / 0	-78.0	-78.0	0.57 (1)	4.22	K-E	0 / 362	0.08 (1)	
D-E	-2073 / 0	-78.0	-78.0	0.57 (1)	4.22	E-J	-1099 / 0	0.41 (1)	
E-F	-819 / 0	-78.0	-78.0	0.48 (1)	6.02	J-F	0 / 827	0.19 (1)	
F-G	-152 / 0	-78.0	-78.0	0.17 (1)	6.25	F-I	-927 / 0	0.59 (1)	
H-G	-1220 / 0	0.0	0.0	0.61 (1)	7.26	I-G	0 / 1071	0.24 (1)	
M-B	-1296 / 0	0.0	0.0	0.13 (1)	7.09	B-L	0 / 2427	0.55 (1)	

M-L	0 / 6	-18.5	-18.5	0.25 (4)	10.00
L-K	0 / 2530	-18.5	-18.5	0.52 (1)	10.00
K-J	0 / 1873	-18.5	-18.5	0.41 (1)	10.00
J-I	0 / 715	-18.5	-18.5	0.27 (4)	10.00
I-H	0 / 0	-18.5	-18.5	0.00 (4)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:	
TOP CH. LL	= 21.0 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 34.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.85")
CALCULATED VERT. DEFL.(LL) = L/999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.85")
CALCULATED VERT. DEFL.(TL) = L/961 (0.32")

CSI: TC=0.82/1.00 (B-C:1) , BC=0.52/1.00 (K-L:1) , WB=0.61/1.00 (C-K:1) , SSI=0.22/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

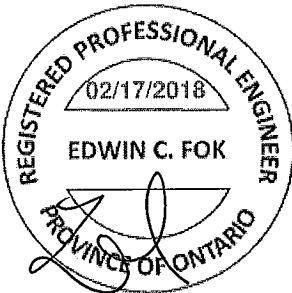
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1667

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (L) (INPUT = 0.90)
JSI METAL= 0.62 (L) (INPUT = 1.00)

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

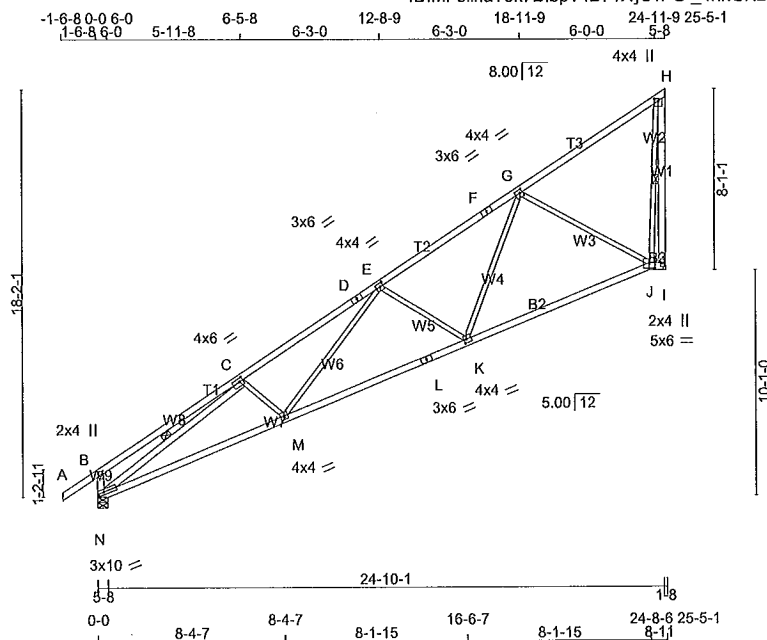


A-18023436

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292589	H7T	8	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 8.200 S Jan 6 2018 MITek Industries, Inc. Sat Feb 17 11:40:14 2018 Page 1
ID:MF8iiriaTok7DlbpT4B71Xys4PG-4nnGK295yp_vQCYJHDxFTyy_sXVWzsNQbuX0pzKFWF



Scale = 1:103.0

TOTAL WEIGHT = 8 X 116 = 925 lb

LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N - L, G, A, RULES				
N - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
I - J	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
N - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)							
JT	TYPE	PLATES	W	LEN	Y	X	
B	TMV+p	MT20	2.0	4.0			
C	TMWW-t	MT20	4.0	6.0	1.75	2.00	
D	TS-t	MT20	3.0	6.0			
E	TMWW-t	MT20	4.0	4.0	2.00	1.75	
F	TS-t	MT20	3.0	6.0			
G	TMWW-t	MT20	4.0	4.0	1.75	1.50	
H	TMVW+p	MT20	4.0	4.0	1.00	2.00	
I	BMV1+p	MT20	2.0	4.0			
J	BBWW-t	MT20	5.0	6.0	2.75	3.00	
K	BMWW-t	MT20	4.0	4.0	2.00	1.25	
L	BS-t	MT20	3.0	6.0			
M	BMWW-t	MT20	4.0	4.0			
N	BMVW1-t	MT20	3.0	10.0	1.50	3.00	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQRD
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG
N	1353	0	DOWN	HORZ	UPLIFT
I	1226	0	1353	0	0

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	962	602 / 0	0 / 0	0 / 0	0 / 0	360 / 0	0 / 0
I	874	534 / 0	0 / 0	0 / 0	0 / 0	341 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.90 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF H-I, C-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MAX. FACTORED		FACTORED			MAX. FACTORED			
MEMB.	FORCE	VERT. LOAD	LC1	MAX	MAX.	MEMB.	FORCE	MAX
	(LBS)	(PLF)		CSI (LC)	UNBRAC		(LBS)	CSI (LC)
FR-TO		FROM	TO		LENGTH	FR-TO		
N-B	-327 / 0	0.0	0.0	0.03 (1)	7.81	N-C	-2962 / 0	0.96 (1)
A-B	0 / 35	-78.0	-78.0	0.15 (1)	10.00	C-M	-139 / 26	0.03 (1)
B-C	0 / 26	-78.0	-78.0	0.41 (1)	10.00	M-E	0 / 521	0.12 (1)
C-D	-2609 / 0	-78.0	-78.0	0.53 (1)	3.90	E-K	-610 / 0	0.23 (1)
D-E	-2609 / 0	-78.0	-78.0	0.53 (1)	3.90	K-G	0 / 906	0.20 (1)
E-F	-1628 / 0	-78.0	-78.0	0.43 (1)	4.76	G-J	-1120 / 0	0.98 (1)
F-G	-1628 / 0	-78.0	-78.0	0.43 (1)	4.76	J-H	0 / 1092	0.25 (1)
G-H	-151 / 0	-78.0	-78.0	0.39 (1)	6.25			
I-H	-1220 / 0	0.0	0.0	0.35 (1)	5.80			

N-M	0 / 2462	-18.5	-18.5	0.56 (1)	10.00
M-L	0 / 2019	-18.5	-18.5	0.49 (1)	10.00
L-K	0 / 2019	-18.5	-18.5	0.49 (1)	10.00
K-J	0 / 1144	-18.5	-18.5	0.39 (4)	10.00
J-I	0 / 0	-18.5	-18.5	0.00 (4)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 23.0 P.S.F. G.S.I. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.85")
CALCULATED VERT. DEFL.(LL) = L/999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.85")
CALCULATED VERT. DEFL.(TL) = L/996 (0.31")

CSI: TC=0.53/1.00 (C-E:1), BC=0.56/1.00 (M-N:1), WB=0.98/1.00 (G-J:1), SSI=0.20/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

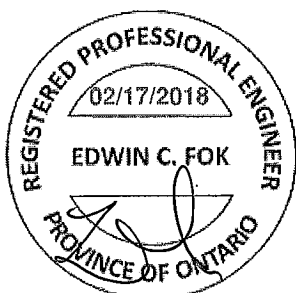
NAIL VALUES	PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)	(PLI)
MT20	618	354	1667	788
			1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

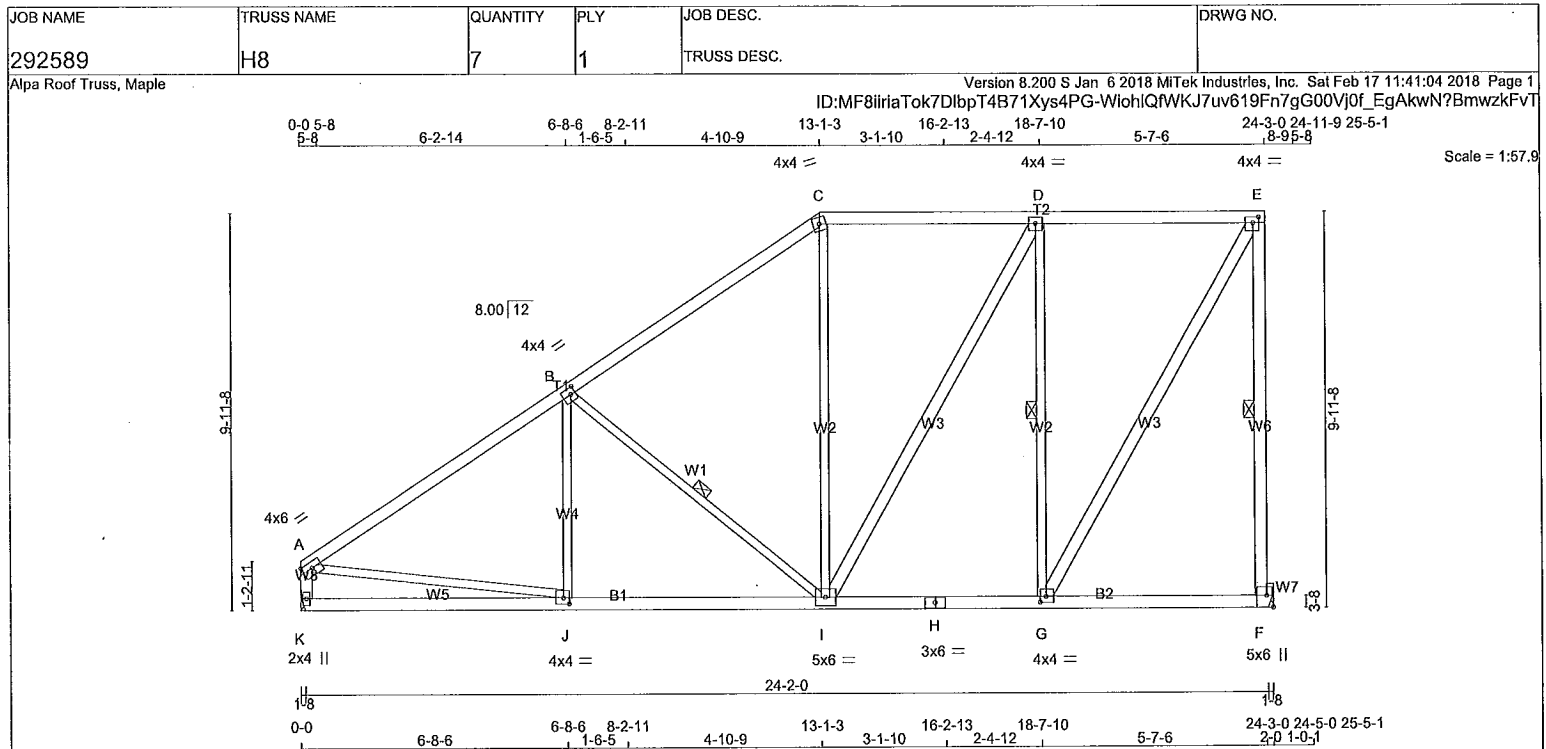
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.73 (C) (INPUT = 1.00)

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



A-18023437



TOTAL WEIGHT = 7 X 126 = 883 lb

(M/F)

LUMBER					DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER			
K - A	2x4	DRY	No.2	SPF		
A - C	2x4	DRY	No.2	SPF		
C - E	2x4	DRY	No.2	SPF		
K - H	2x4	DRY	No.2	SPF		
H - F	2x4	DRY	No.2	SPF		
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF		
F - E	2x4	DRY	No.2	SPF		
I - D	2x4	DRY	No.2	SPF		
G - E	2x4	DRY	No.2	SPF		

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-1	MT20	4.0	6.0	1.75	Edge
B	TMVW-1	MT20	4.0	4.0	2.00	1.50
C	TTW-m	MT20	4.0	4.0		
D	TMVW-1	MT20	4.0	4.0		
E	TMVW-1	MT20	4.0	4.0	1.75	1.75
F	BMVW1+1	MT20	5.0	6.0	3.50	Edge
G	BMVW-1	MT20	4.0	4.0	1.75	1.75
H	BS-1	MT20	3.0	6.0		
I	BMVWVW-1	MT20	5.0	6.0		
J	BMVW-1	MT20	4.0	4.0	1.75	1.75
K	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT	REQD
	VERT	HORZ	DOWN	HORZ		
K	1181	0	1181	0	0	BRG
F	1226	0	1226	0	0	BRG

HANGER BY OTHERS
MIN. SEAT SIZE: 1-8
HANGER BY OTHERS
MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM	LIVE			
K	844	506 / 0	0 / 0	0 / 0	0 / 0	338 / 0	0 / 0
F	880	506 / 0	0 / 0	0 / 0	0 / 0	374 / 0	0 / 0

BRACING

FOR SECTION C-E, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.96 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-F, D-G, B-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS (LC)	
FR-TO		FROM TO		FR-TO			
K-A	-1131 / 0	0.0	0.0 0.11 (1)	F-E	-1185 / 0	0.57 (1)	
A-B	-1362 / 0	-78.0	-78.0 0.50 (1)	I-C	0 / 132	0.04 (4)	
B-C	-933 / 0	-78.0	-78.0 0.45 (1)	I-D	0 / 361	0.06 (1)	
C-D	-748 / 0	-85.5	-85.5 0.34 (1)	G-D	-897 / 0	0.60 (1)	
D-E	-571 / 0	-85.5	-85.5 0.33 (1)	G-E	0 / 1159	0.19 (1)	
				A-J	0 / 1172	0.26 (1)	
K-J	0 / 0	-18.5	-18.5 0.20 (4)	J-B	-18 / 94	0.03 (4)	
J-I	0 / 1162	-18.5	-18.5 0.31 (4)	B-I	-538 / 0	0.26 (1)	
I-H	0 / 571	-18.5	-18.5 0.19 (4)				
H-G	0 / 571	-18.5	-18.5 0.19 (4)				
G-F	0 / 0	-18.5	-18.5 0.13 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	34.4	PSF	

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(65 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.80")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.80")
CALCULATED VERT. DEFL.(TL) = L/999 (0.09")

CSI: TC=0.50/1.00 (A-B:1), BC=0.31/1.00 (I-J:4), WB=0.60/1.00 (D-G:1), SSI=0.23/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)	(PLI)
	MAX	MIN	MAX	MIN
	618	354	1667	788
			1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

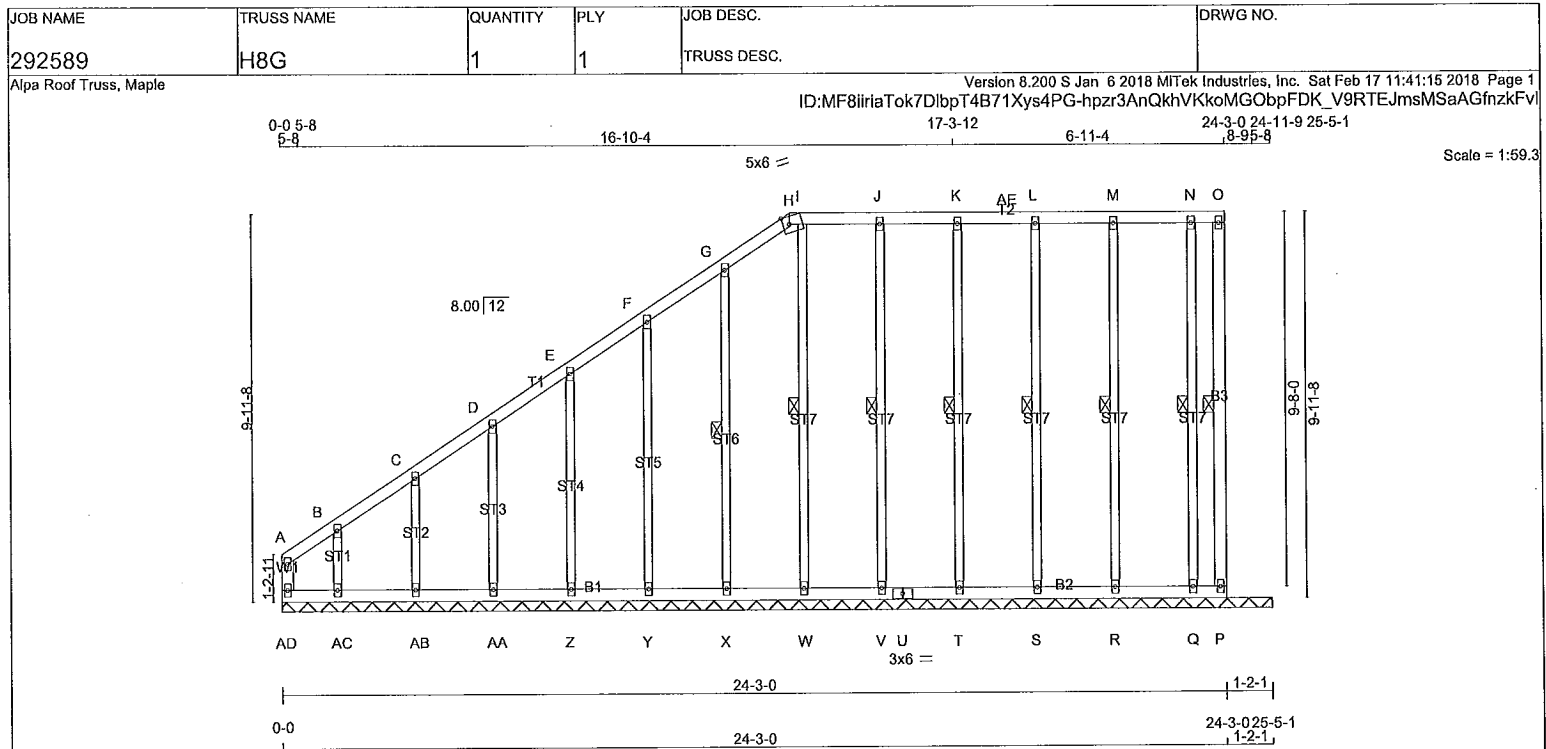
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (J) (INPUT = 0.90)
JSI METAL= 0.44 (J) (INPUT = 1.00)

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



A-18023438



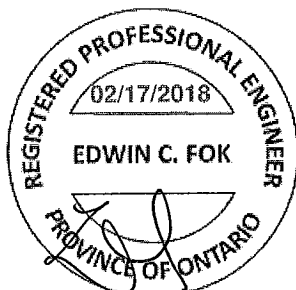
TOTAL WEIGHT = 140 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
AD- A	2x4 DRY	No.2	SPF	
A - H	2x4 DRY	No.2	SPF	
H - O	2x4 DRY	No.2	SPF	
AD- U	2x4 DRY	No.2	SPF	
U - P	2x4 DRY	No.2	SPF	
P - O	2x4 DRY	No.2	SPF	
ALL WEBS	2x3 DRY	No.2	SPF	
EXCEPT				
ALL GABLE WEBS	2x3 DRY	No.2	SPF	
DRY: SEASONED LUMBER.				
GABLE STUDS SPACED AT 2-0-0 OC.				

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
A	TMV+p	MT20	2.0	4.0
B, C, D, E, F, G, J, K, L, M, N	TMV+w	MT20	2.0	4.0
H	TTW-m	MT20	5.0	6.0 Edge 2.00
I	TMV+p	MT20	2.0	4.0
O	BMV1+p	MT20	2.0	4.0
P	BMV1+p	MT20	2.0	4.0
Q, R, S, T, V, W, X, Y, Z, AA, AB, AC	BMV1+w	MT20	2.0	4.0
U	BS-t	MT20	3.0	6.0
AD	BMV1+p	MT20	2.0	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

FOR SECTION H-O, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF O-P, N-Q, M-R, L-S, K-T, J-V, I-W, G-X.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRACED LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO					
AD-A	-58 / 0	0.0	0.0 0.01 (1)	7.81	Q-N	-111 / 0	0.07 (1)
A-B	-19 / 0	-78.0	-78.0 0.03 (1)	6.25	R-M	-182 / 0	0.12 (1)
B-C	-14 / 0	-78.0	-78.0 0.04 (1)	6.25	S-L	-170 / 0	0.11 (1)
C-D	-11 / 0	-78.0	-78.0 0.04 (1)	6.25	T-K	-171 / 0	0.11 (1)
D-E	-7 / 0	-78.0	-78.0 0.04 (1)	10.00	V-J	-171 / 0	0.11 (1)
E-F	-5 / 0	-78.0	-78.0 0.04 (1)	10.00	W-I	-169 / 0	0.11 (1)
F-G	-2 / 0	-78.0	-78.0 0.04 (1)	10.00	X-G	-158 / 0	0.08 (1)
G-H	-8 / 0	-78.0	-78.0 0.04 (1)	10.00	Y-F	-154 / 0	0.16 (1)
H-I	0 / 2	-85.5	-85.5 0.04 (1)	2.00	Z-E	-155 / 0	0.09 (1)
I-J	0 / 0	-85.5	-85.5 0.04 (1)	2.00	AA-D	-153 / 0	0.05 (1)
J-K	0 / 0	-85.5	-85.5 0.04 (1)	2.00	AB-C	-158 / 0	0.03 (1)
K-AE	0 / 0	-85.5	-85.5 0.04 (1)	2.00	AC-B	-130 / 0	0.02 (1)
AE-L	0 / 0	-85.5	-85.5 0.04 (1)	2.00			
L-M	0 / 0	-85.5	-85.5 0.04 (1)	2.00			
M-N	0 / 0	-85.5	-85.5 0.04 (1)	2.00			
N-O	0 / 0	-85.5	-85.5 0.01 (1)	2.00			

AD-AC	0 / 19	-18.5	-18.5 0.01 (4)	10.00
AC-AB	0 / 13	-18.5	-18.5 0.02 (4)	10.00
AB-AA	0 / 9	-18.5	-18.5 0.02 (4)	10.00
AA-Z	0 / 6	-18.5	-18.5 0.01 (4)	10.00
Z-Y	0 / 4	-18.5	-18.5 0.01 (4)	10.00
Y-X	0 / 2	-18.5	-18.5 0.01 (4)	10.00
X-W	0 / 0	-18.5	-18.5 0.01 (4)	10.00
W-V	0 / 0	-18.5	-18.5 0.01 (4)	10.00
V-U	0 / 0	-18.5	-18.5 0.01 (4)	10.00
U-T	0 / 0	-18.5	-18.5 0.01 (4)	10.00
T-S	0 / 0	-18.5	-18.5 0.01 (4)	10.00
S-R	0 / 0	-18.5	-18.5 0.02 (4)	10.00
R-Q	0 / 0	-18.5	-18.5 0.02 (4)	10.00
Q-P	0 / 0	-18.5	-18.5 0.01 (4)	10.00
P-O	-22 / 0	0.0	0.0 0.01 (1)	6.25

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	34.4	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(5% OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.04/1.00 (L-M:1), BC=0.02/1.00 (AB-AC:4), WB=0.16/1.00 (F-Y:1), SSI=0.07/1.00 (M-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

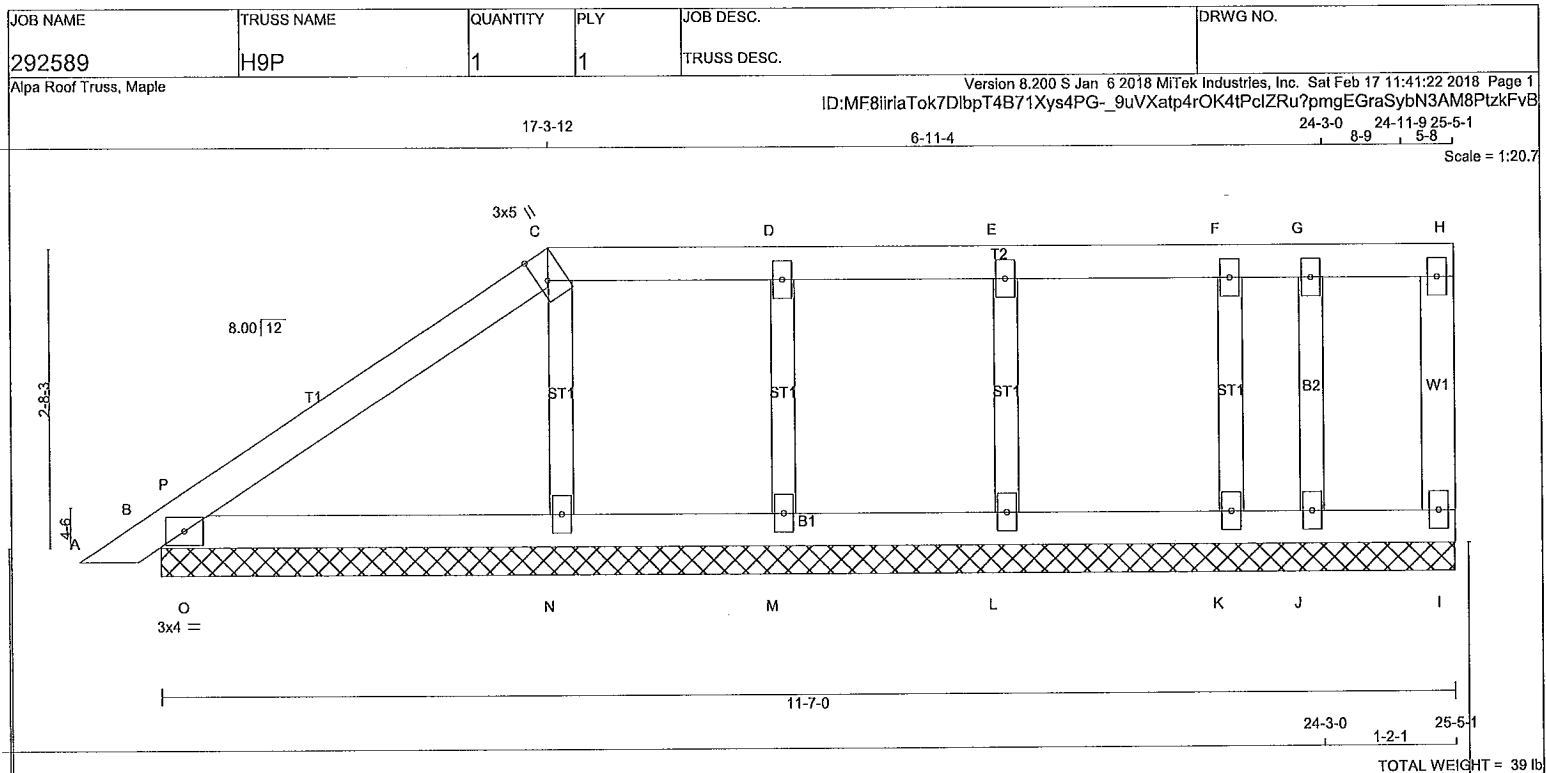
PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 788	1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.40 (H) (INPUT = 0.90)
JSI METAL= 0.08 (G) (INPUT = 1.00)

A-18023439



LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
J - G	2x3	DRY	No.2	SPF
B - I	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				
GABLE STUDS SPACED AT 2-0-0 OC.				

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
B TMB1-I	MT20	3.0	4.0	
C TTW+h	MT20	3.0	5.0	Edge 1.00
D, E, F				
D TMW+w	MT20	2.0	4.0	
G TMV+p	MT20	2.0	4.0	
H TMV+p	MT20	2.0	4.0	
I BMV1+p	MT20	2.0	4.0	
J BMV1+p	MT20	2.0	4.0	
K, L, M, N				
K BMV1+w	MT20	2.0	4.0	
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.				

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LC1 (LC)	UNBRACED LENGTH (FT)
FR-TO		FROM TO		FR-TO			
A-B	0 / 13	-78.0 -78.0	0.02 (1)	10.00	K-F	-115 / 0	0.02 (1)
B-P	-9 / 42	-78.0 -78.0	0.04 (1)	10.00	L-E	-155 / 0	0.03 (1)
P-C	-15 / 0	-78.0 -78.0	0.12 (1)	6.25	M-D	-181 / 0	0.03 (1)
C-D	0 / 0	-78.0 -78.0	0.05 (1)	10.00	N-C	-176 / 0	0.03 (1)
D-E	0 / 0	-78.0 -78.0	0.05 (1)	10.00	O-P	-227 / 0	0.00 (1)
E-F	0 / 0	-78.0 -78.0	0.03 (1)	10.00			
F-G	0 / 0	-78.0 -78.0	0.02 (1)	10.00			
G-H	0 / 0	-78.0 -78.0	0.02 (1)	10.00			
H-I	-42 / 0	0.0 0.0	0.01 (1)	7.81			
J-G	-73 / 0	0.0 0.0	0.01 (1)	7.81			
B-O	0 / 8	-18.5 -18.5	0.10 (1)	10.00			
O-N	0 / 8	-18.5 -18.5	0.10 (1)	10.00			
N-M	0 / 0	-18.5 -18.5	0.06 (1)	10.00			
M-L	0 / 0	-18.5 -18.5	0.02 (4)	10.00			
L-K	0 / 0	-18.5 -18.5	0.02 (4)	10.00			
K-J	0 / 0	-18.5 -18.5	0.01 (4)	10.00			
J-I	0 / 0	-18.5 -18.5	0.01 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.12/1.00 (C-P:1), BC=0.10/1.00 (B-O:1), WB=0.03/1.00 (D-M:1), SSI=0.18/1.00 (B-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

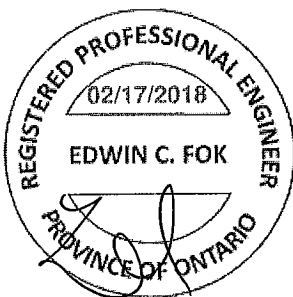
NAIL VALUES	PLATE GRIP (DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 788	1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (B) (INPUT = 0.90)
JSI METAL= 0.05 (C) (INPUT = 1.00)

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



A-18023440

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292589	H11P	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 8.200 S Jan 6 2018 MITek Industries, Inc. Sat Feb 17 11:42:12 2018 Page 1
ID:MF8lriaTok7DlbpT4B71Xys4PG-XnvO1gTAJChE3ZDDE3Lc1Mr95QupAYtkZyUn9zzkFuP

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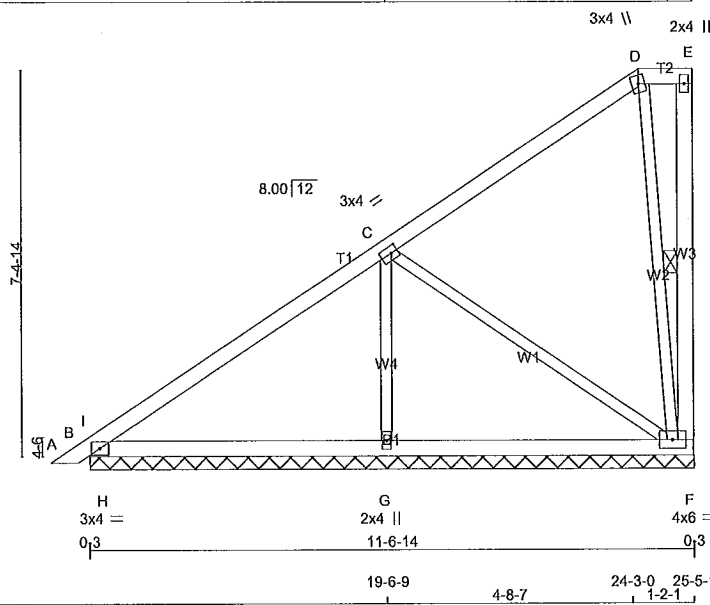
19-6-9

5-5-0

24-11-9 25-5-1

5-8

Scale = 1:44.2



19-6-9

19-6-9

4-8-7

24-3-0 25-5-1

1-2-1

TOTAL WEIGHT = 51 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	DRY	LUMBER
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
F - E	2x4	DRY	No.2
B - F	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB-I	MT20	3.0	4.0	
C	TMWW-I	MT20	3.0	4.0	1.50 1.50
D	TTW+m	MT20	3.0	4.0	
E	TMV+p	MT20	2.0	4.0	
F	BMVWW-I	MT20	4.0	6.0	
G	BMV-I+w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
F	305	0	305	0	11-7-1 (11-6-12-15)	
B	349	0	349	0	11-7-1 (11-6-12-15)	
G	515	0	515	0	11-7-1 (11-6-12-15)	

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS							
JT	1ST CASE	MAX	MIN	COMPONENT REACTIONS			
F	216	140 / 0	0 / 0	PERM. LIVE	WIND	DEAD	SOIL
B	246	164 / 0	0 / 0	0 / 0	0 / 0	82 / 0	0 / 0
G	370	209 / 0	0 / 0	0 / 0	0 / 0	161 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

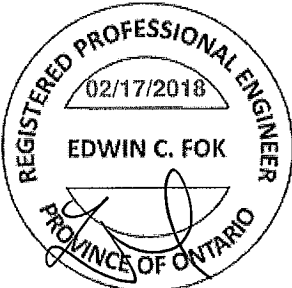
1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX LC1 (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 13	-78.0	-78.0 0.02 (1)	10.00	G-C	-355 / 0	0.09 (1)
B-I	-59 / 0	-78.0	-78.0 0.07 (1)	6.25	D-F	-164 / 0	0.16 (1)
I-C	-129 / 0	-78.0	-78.0 0.34 (1)	6.25	C-F	-120 / 0	0.10 (1)
C-D	-57 / 0	-78.0	-78.0 0.34 (1)	6.25	H-I	-329 / 0	0.00 (1)
D-E	0 / 0	-78.0	-78.0 0.01 (1)	10.00			
F-E	-40 / 0	0.0	0.0 0.01 (1)	6.25			
B-H	0 / 124	-18.5	-18.5 0.17 (1)	10.00			
H-G	0 / 124	-18.5	-18.5 0.19 (4)	10.00			
G-F	0 / 124	-18.5	-18.5 0.19 (4)	10.00			

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.34/1.00 (C-I:1), BC=0.19/1.00 (G-H:4), WB=0.16/1.00 (D-F:1), SSI=0.27/1.00 (B-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	618	354	1667 788 1987 1656

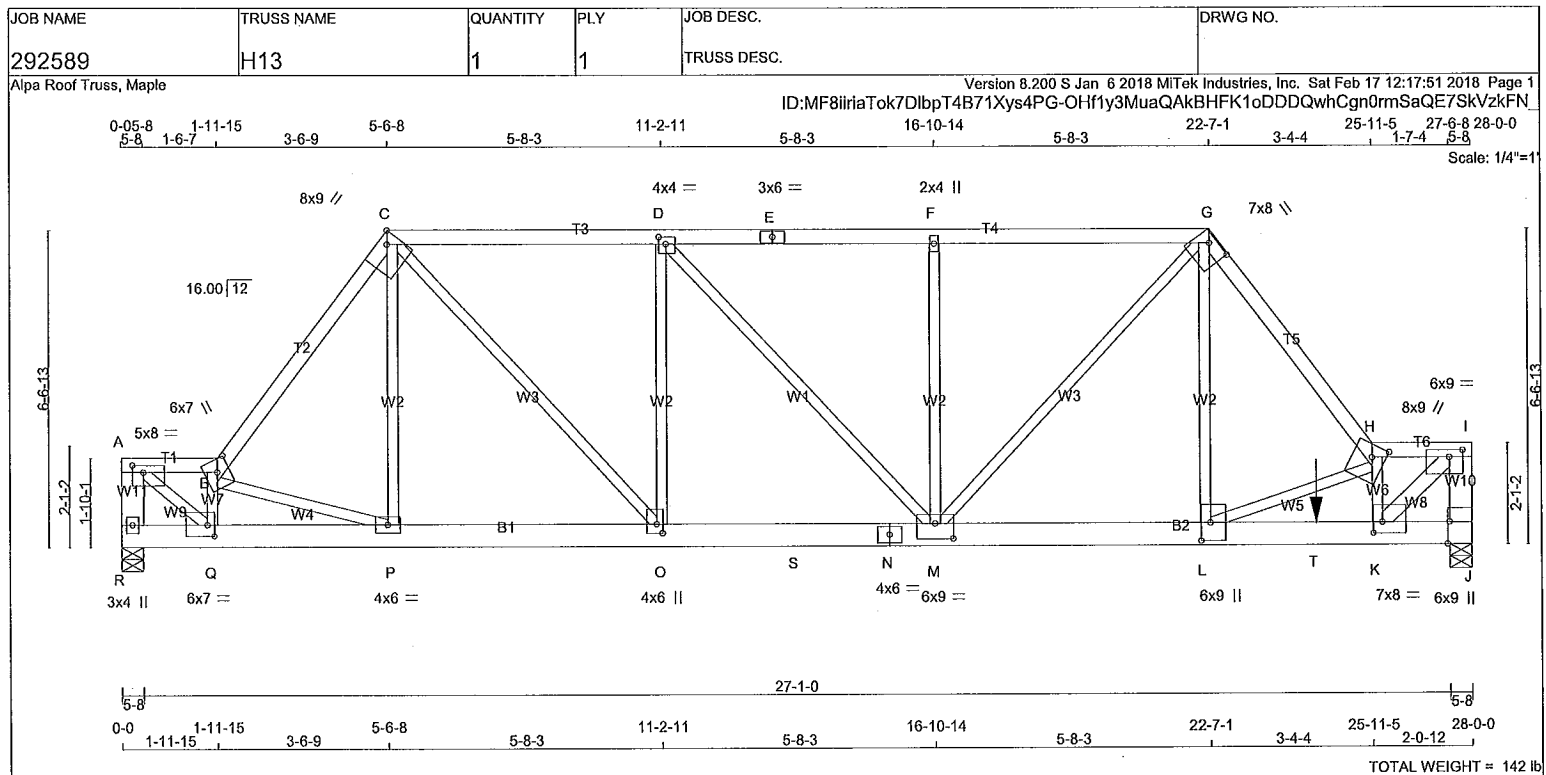
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.44 (C) (INPUT = 0.90)
JSI METAL= 0.08 (C) (INPUT = 1.00)

A-18023442

A-18023443



LUMBER				N. L. G. A. RULES	
CHORDS	SIZE	LUMBER	DESCR.		
R - A	2x6	DRY	No.2	SPF	
A - B	2x4	DRY	No.2	SPF	
B - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	2100F 1.8E	SPF	
E - G	2x4	DRY	2100F 1.8E	SPF	
G - H	2x4	DRY	1650F 1.5E	SPF	
H - I	2x4	DRY	No.2	SPF	
J - I	2x6	DRY	No.2	SPF	
R - N	2x6	DRY	1650F 1.5E	SPF	
N - J	2x6	DRY	1650F 1.5E	SPF	
ALL WEBS EXCEPT K - I	2x4	DRY	1650F 1.5E	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW-t	MT20	5.0	8.0	1.75 2.75
B	TTWW+m	MT20	6.0	7.0	3.25 3.00
C	TTWW-h	MT20	8.0	9.0	Edge 2.75
D	TMVW-t	MT20	4.0	4.0	1.75 1.75
E	TS-t	MT20	3.0	6.0	
F	TMVW+m	MT20	2.0	4.0	
G	TTWW-h	MT20	7.0	8.0	1.75 5.00
H	TTWW+m	MT20	8.0	9.0	3.00 3.25
I	TMVW-t	MT20	6.0	9.0	1.75 3.25
J	BMV1+t	MT20	6.0	9.0	Edge 0.50
K	BMVW-t	MT20	7.0	8.0	2.75 2.25
L	BMVW-t	MT20	6.0	9.0	4.50 2.25
M	BMVWVW-t	MT20	6.0	9.0	3.75 4.50
N	BS-t	MT20	4.0	6.0	
O	BMVW-t	MT20	4.0	6.0	2.25 1.50
P	BMVW-t	MT20	4.0	6.0	
Q	BMVW-t	MT20	6.0	7.0	2.75 1.75
R	BMV1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ UPLIFT
R	2285	0	2285	0
J	3974	0	3974	0

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1629	995 / 0	0 / 0	0 / 0	0 / 0	635 / 0	0 / 0
J	2833	1729 / 0	0 / 0	0 / 0	0 / 0	1104 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J
BEARING SIZE FACTOR = 1.15 AT JNT(S) J (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.08 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED MAX CSI (LC)	
FR-TO		FROM TO			FR-TO			
R-A	-2081 / 0	0.0	0.0	0.15 (1)	7.00	A-Q	0 / 3374	0.84 (1)
A-B	-2867 / 0	-78.0	-78.0	0.15 (1)	4.10	Q-B	-2322 / 0	0.36 (1)
B-C	-3037 / 0	-78.0	-78.0	0.41 (1)	3.68	B-P	-1088 / 0	0.31 (1)
C-D	-3218 / 0	-78.0	-78.0	0.34 (1)	4.42	P-C	0 / 586	0.14 (1)
D-E	-3970 / 0	-78.0	-78.0	0.39 (1)	4.02	C-O	0 / 2041	0.51 (1)
E-F	-3970 / 0	-78.0	-78.0	0.39 (1)	4.02	O-D	-1281 / 0	0.86 (1)
F-G	-3970 / 0	-78.0	-78.0	0.36 (1)	4.04	D-M	0 / 1112	0.28 (1)
G-H	-4928 / 0	-78.0	-78.0	0.37 (1)	3.45	M-F	-444 / 0	0.30 (1)
H-I	-4447 / 0	-78.0	-78.0	0.31 (1)	3.08	M-G	0 / 1428	0.35 (1)
J-I	-3886 / 0	0.0	0.0	0.29 (1)	5.39	L-G	0 / 2547	0.63 (1)
						L-H	-1892 / 0	0.51 (1)
R-Q	0 / 0	-18.5	-18.5	0.18 (1)	10.00	K-H	-3442 / 0	0.55 (1)
Q-P	0 / 2836	-18.5	-18.5	0.41 (1)	10.00	K-I	0 / 5921	0.76 (1)
P-O	0 / 1832	-18.5	-18.5	0.21 (1)	10.00			
O-S	0 / 3218	-18.5	-18.5	0.56 (1)	10.00			
S-N	0 / 3218	-271.7	-271.7	0.56 (1)	10.00			
N-M	0 / 3218	-271.7	-271.7	0.56 (1)	10.00			
M-L	0 / 3000	-271.7	-271.7	0.59 (1)	10.00			
L-T	0 / 4660	-271.7	-271.7	0.78 (1)	10.00			
T-K	0 / 4660	-18.5	-18.5	0.78 (1)	10.00			
K-J	0 / 0	-18.5	-18.5	0.07 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
T	24-9-4	-830	-830	--	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL = 21.0	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CSIdGirder
START DISTANCE = 14-0-0
START SPAN CARRIED = 12-6-0
END DISTANCE = 24-9-4
END SPAN CARRIED = 12-6-0
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDTL LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.93")
CALCULATED VERT. DEFL.(LL)= L/999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.93")
CALCULATED VERT. DEFL.(TL)= L/999 (0.30")

CSI: TC=0.41/1.00 (B-C:1), BC=0.78/1.00 (K-L:1), WB=0.86/1.00 (D-O:1), SSI=0.63/1.00 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

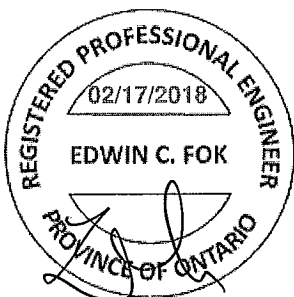
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

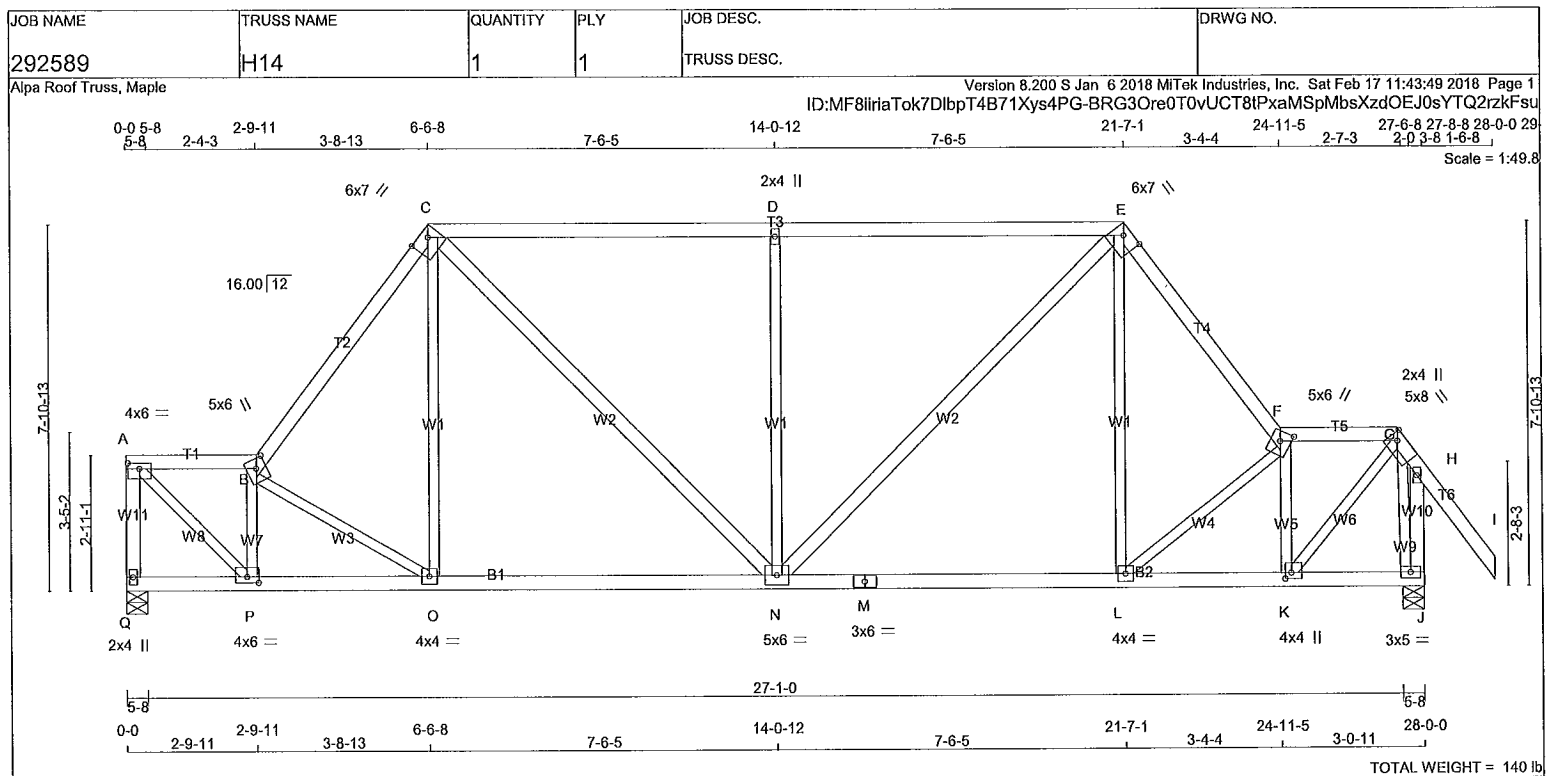
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	618	354	1667
	1667	788	1987

A-18023444

CONTINUED ON PAGE 2





LUMBER

N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
Q - A	2x4 DRY	No.2	SPF
A - B	2x4 DRY	No.2	SPF
B - C	2x4 DRY	No.2	SPF
C - E	2x4 DRY	No.2	SPF
E - F	2x4 DRY	No.2	SPF
F - G	2x4 DRY	No.2	SPF
G - I	2x4 DRY	No.2	SPF
J - H	2x4 DRY	No.2	SPF
Q - M	2x4 DRY	No.2	SPF
M - J	2x4 DRY	No.2	SPF
ALL WEBS	2x3 DRY	No.2	SPF
EXCEPT			
C - N	2x4 DRY	No.2	SPF
N - E	2x4 DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0	1.50	3.00
B	TTWW+m	MT20	5.0	6.0	2.50	2.50
C	TTWW-h	MT20	6.0	7.0	2.00	4.25
D	TMVW+m	MT20	2.0	4.0		
E	TTWW-h	MT20	6.0	7.0	2.00	4.25
F	TTWW+m	MT20	5.0	6.0	2.50	2.50
G	TTWW-h	MT20	5.0	8.0	2.00	2.00
H	TMV+p	MT20	2.0	4.0		
J	BMVW1-t	MT20	3.0	5.0		
K	BMVW1-t	MT20	4.0	4.0	1.50	1.50
L	BMVW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMVW1-t	MT20	5.0	6.0		
O	BMVW-t	MT20	4.0	4.0		
P	BMVW-t	MT20	4.0	6.0	1.50	3.00
Q	BMV1+p	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	UP
Q	1347	0	1347	0
J	1484	0	1484	0

UNFACTORED REACTIONS

	1ST LOASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	COMBINED						
Q	960	586 / 0	0 / 0	0 / 0	0 / 0	375 / 0	0 / 0
J	1055	659 / 0	0 / 0	0 / 0	0 / 0	396 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.10 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM	TO	FR-TO			
Q-A	-1313 / 0	0.0	0.0 0.19 (1)	7.05	A-P	0 / 1757	0.40 (1)
A-B	-1259 / 0	-78.0	-78.0 0.12 (1)	5.60	P-B	-1195 / 0	0.21 (1)
B-C	-1493 / 0	-78.0	-78.0 0.25 (1)	5.13	B-O	-495 / 0	0.17 (1)
C-D	-1318 / 0	-78.0	-78.0 0.90 (1)	4.10	O-C	0 / 400	0.09 (1)
D-E	-1318 / 0	-78.0	-78.0 0.90 (1)	4.10	C-N	0 / 592	0.10 (1)
E-F	-1460 / 0	-78.0	-78.0 0.20 (1)	5.22	N-D	-714 / 0	0.84 (1)
F-G	-1117 / 0	-78.0	-78.0 0.10 (1)	5.89	D-E	0 / 620	0.10 (1)
G-H	-157 / 0	-78.0	-78.0 0.15 (1)	6.25	E-F	0 / 368	0.08 (1)
H-I	0 / 52	-78.0	-78.0 0.15 (1)	10.00	L-F	-375 / 0	0.13 (1)
J-H	-343 / 0	0.0	0.0 0.05 (1)	7.81	K-F	-1147 / 0	0.23 (1)
					K-G	0 / 1495	0.34 (1)
					G-J	-1127 / 0	0.21 (1)
Q-P	0 / 0	-18.5	-18.5 0.04 (4)	10.00			
P-O	0 / 1306	-18.5	-18.5 0.30 (1)	10.00			
O-N	0 / 901	-18.5	-18.5 0.31 (4)	10.00			
N-M	0 / 881	-18.5	-18.5 0.31 (4)	10.00			
M-L	0 / 881	-18.5	-18.5 0.31 (4)	10.00			
L-K	0 / 1156	-18.5	-18.5 0.27 (1)	10.00			
K-J	0 / 201	-18.5	-18.5 0.07 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 21.0 PSF
DL = 6.0 PSF	
BOT CH.	LL = 0.0 PSF
DL = 7.4 PSF	
TOTAL LOAD = 34.4 PSF	

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(5% OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.93")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.93")
CALCULATED VERT. DEFL.(TL) = L/999 (0.14")

CSI: TC=0.90/1.00 (C-D:1), BC=0.31/1.00 (N-O:4),
WB=0.84/1.00 (D-N:1), SSI=0.29/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667
		788	1987
			1656

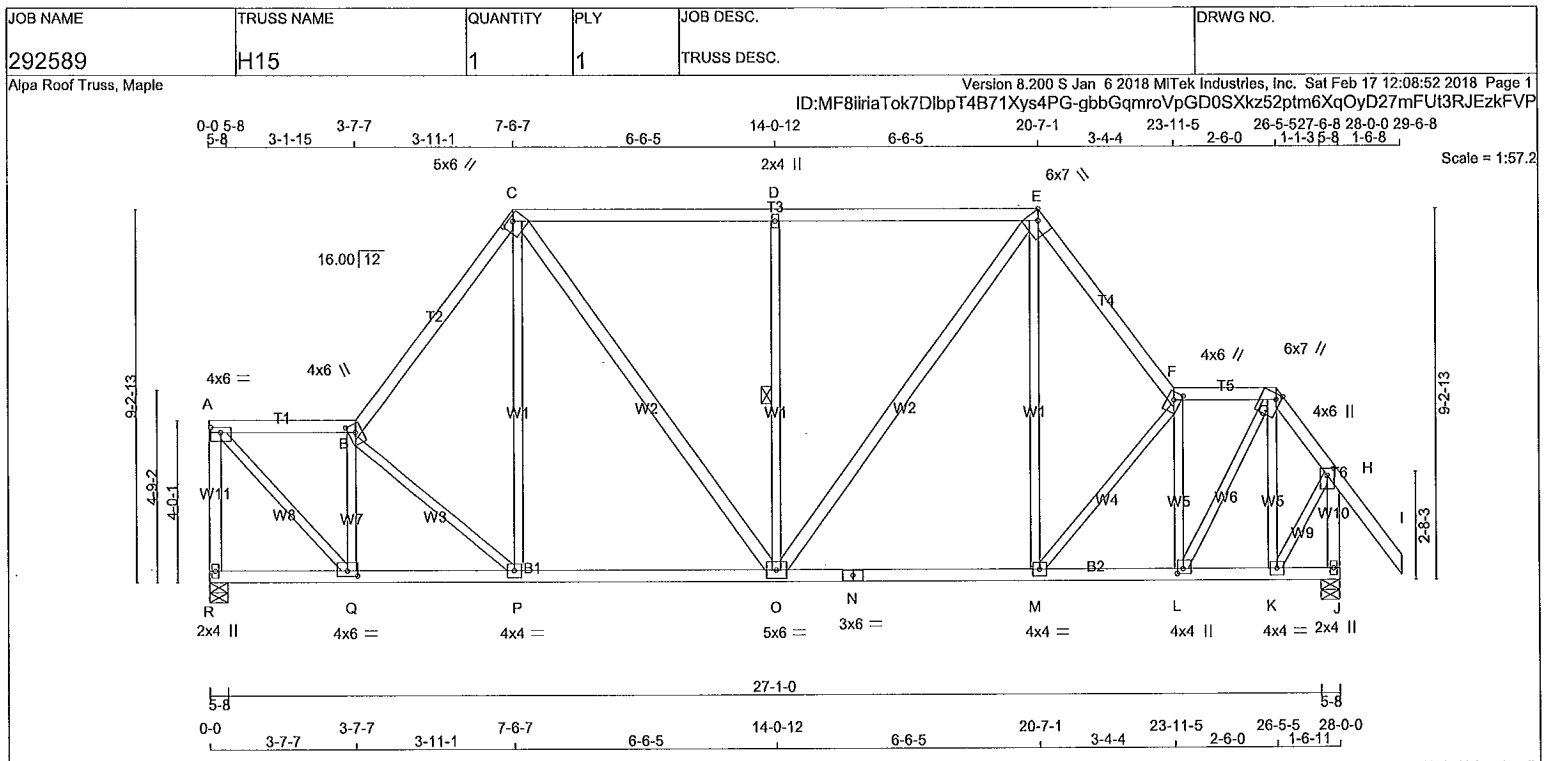
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (G) (INPUT = 0.90)
JSI METAL= 0.42 (K) (INPUT = 1.00)

A-18023445





TOTAL WEIGHT = 155 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
R - A	2x4	DRY	No.2
A - B	2x4	DRY	No.2
B - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
J - H	2x4	DRY	No.2
R - N	2x4	DRY	No.2
N - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
C - O	2x4	DRY	No.2
O - E	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
A	TMVW-t	MT20	4.0	6.0 1.50 3.00
B	TTWW+m	MT20	4.0	6.0 2.50 2.00
C	TTWW-h	MT20	5.0	6.0 1.25 3.00
D	TMVW+w	MT20	2.0	4.0
E	TTWW-h	MT20	6.0	7.0 Edge 2.75
F	TTWW+m	MT20	4.0	6.0 2.25 2.00
G	TTWW+m	MT20	6.0	7.0 Edge 1.50
H	TMVW+p	MT20	4.0	6.0 2.00 2.00
J	BMV1+p	MT20	2.0	4.0
K, M, P				
K	BMWW-t	MT20	4.0	4.0
L	BMWW+t	MT20	4.0	4.0 1.50 1.75
N	BS-t	MT20	3.0	6.0
O	BMWWW-t	MT20	5.0	6.0
Q	BMWW-t	MT20	4.0	6.0 1.50 3.00
R	BMV1+p	MT20	2.0	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
R	1347	0	1347	0	0
J	1484	0	1484	0	0

UNFACTORED REACTIONS		1ST LCASE	MAX/MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
JT	COMBINED	SNOW	LIVE	PERM.LIVE		
R	960	586 / 0	0 / 0	0 / 0	375 / 0	0 / 0
J	1055	659 / 0	0 / 0	0 / 0	396 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.05 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)
FR-TO		FROM TO		FR-TO		FROM TO	
R-A	-1314 / 0	0.0	0.0 0.33 (1)	A-Q	0 / 1635	0.37 (1)	
A-B	-1124 / 0	-78.0	-78.0 0.19 (1)	Q-B	-1129 / 0	0.28 (1)	
B-C	-1391 / 0	-78.0	-78.0 0.27 (1)	B-P	-435 / 0	0.21 (1)	
C-D	-1104 / 0	-78.0	-78.0 0.63 (1)	P-C	0 / 398	0.09 (1)	
D-E	-1104 / 0	-78.0	-78.0 0.63 (1)	C-O	0 / 448	0.07 (1)	
E-F	-1367 / 0	-78.0	-78.0 0.19 (1)	O-D	-616 / 0	0.34 (1)	
F-G	-1010 / 0	-78.0	-78.0 0.09 (1)	D-E	0 / 473	0.08 (1)	
G-H	-812 / 0	-78.0	-78.0 0.15 (1)	E-H	0 / 381	0.09 (1)	
H-I	0 / 52	-78.0	-78.0 0.15 (1)	I-F	-349 / 0	0.18 (1)	
J-H	-1486 / 0	0.0	0.0 0.21 (1)	F-G	-1047 / 0	0.35 (1)	
R-Q	0 / 0	-18.5	-18.5 0.05 (4)	G-K	0 / 1208	0.27 (1)	
Q-P	0 / 1156	-18.5	-18.5 0.25 (1)	K-H	-542 / 0	0.18 (1)	
P-O	0 / 839	-18.5	-18.5 0.25 (4)				
O-N	0 / 825	-18.5	-18.5 0.25 (4)				
N-M	0 / 825	-18.5	-18.5 0.25 (4)				
M-L	0 / 1035	-18.5	-18.5 0.23 (1)				
L-K	0 / 434	-18.5	-18.5 0.11 (1)				
K-J	0 / 0	-18.5	-18.5 0.04 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD.

ALLOWABLE DEFL.(LL)= L/360 (0.93")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.93")
CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CSI: TC=0.63/1.00 (C-D:1), BC=0.25/1.00 (O-P:4), WB=0.37/1.00 (A-Q:1), SSI=0.25/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

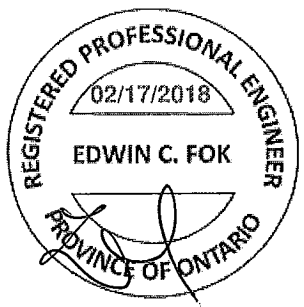
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

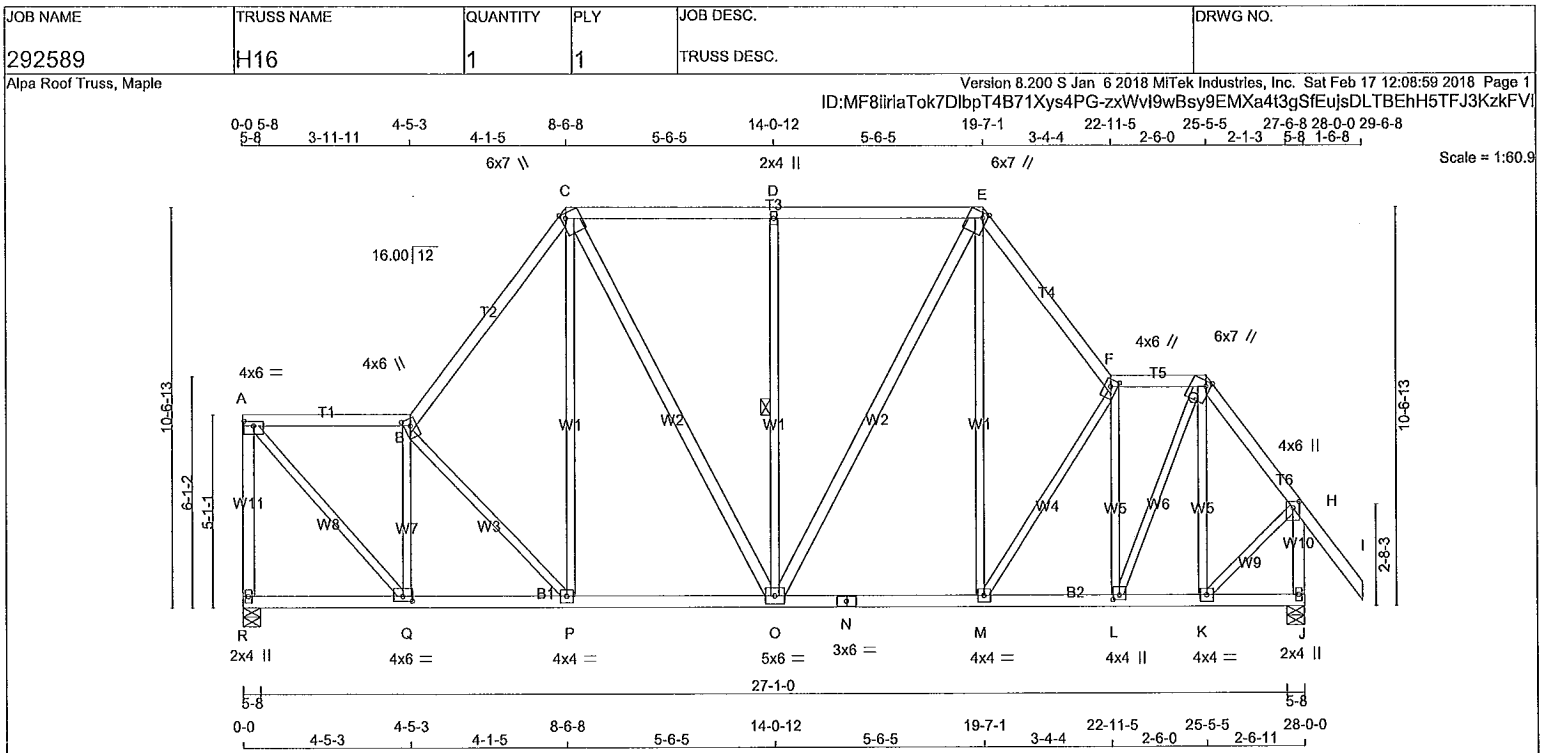
NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	618	354	1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.35 (B) (INPUT = 1.00)





TOTAL WEIGHT = 169 lb

LUMBER		N. L. G. A. RULES		CHORDS		SIZE		LUMBER		DESCR.	
R - A	2x4	DRY	No.2	SPF							
A - B	2x4	DRY	No.2	SPF							
B - C	2x4	DRY	No.2	SPF							
C - E	2x4	DRY	No.2	SPF							
E - F	2x4	DRY	No.2	SPF							
F - G	2x4	DRY	No.2	SPF							
G - I	2x4	DRY	No.2	SPF							
J - H	2x4	DRY	No.2	SPF							
R - N	2x4	DRY	No.2	SPF							
N - J	2x4	DRY	No.2	SPF							
ALL WEBS	2x3	DRY	No.2	SPF							
EXCEPT											
C - O	2x4	DRY	No.2	SPF							
O - E	2x4	DRY	No.2	SPF							

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0	1.50	3.00
B	TTWW+m	MT20	4.0	6.0	2.25	2.00
C, E, G						
C	TTWW+m	MT20	6.0	7.0	Edge	1.50
D	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	4.0	6.0	2.25	2.00
H	TMVW+p	MT20	4.0	6.0	2.00	2.00
J	BMV1+p	MT20	2.0	4.0		
K, M, P						
K	BMWW-t	MT20	4.0	4.0		
L	BMWW-t	MT20	4.0	4.0	1.50	2.00
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	5.0	6.0		
Q	BMWW-t	MT20	4.0	6.0	1.50	3.00
R	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX		
R	1351	0	1351	0	5-8	2-0		
J	1480	0	1480	0	5-8	2-3		

UNFACTORED REACTIONS

JT	1ST LCASE		MAX / MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
R	963	588 / 0	0 / 0	0 / 0	0 / 0	375 / 0	0 / 0
J	1052	657 / 0	0 / 0	0 / 0	0 / 0	395 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.35 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		W E B S		FACTORED	
	MAX. FORCE (LBS)	VERT. LOAD (PLF)	MAX. VERT. LOAD (LC1)	MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO				FR-TO		
R-A	-1315 / 0	0.0 0.0 0.58 (1)	7.05	A-Q	0 / 1553	0.35 (1)		
A-B	-1040 / 0	-78.0 -78.0 0.28 (1)	5.79	Q-B	-1072 / 0	0.41 (1)		
B-C	-1305 / 0	-78.0 -78.0 0.29 (1)	5.35	B-P	-423 / 0	0.30 (1)		
C-D	-953 / 0	-78.0 -78.0 0.43 (1)	5.74	P-C	0 / 405	0.09 (1)		
D-E	-953 / 0	-78.0 -78.0 0.43 (1)	5.74	C-O	0 / 349	0.06 (1)		
E-F	-1294 / 0	-78.0 -78.0 0.19 (1)	5.48	O-D	-518 / 0	0.41 (1)		
F-G	-953 / 0	-78.0 -78.0 0.09 (1)	6.25	D-E	0 / 363	0.06 (1)		
G-H	-940 / 0	-78.0 -78.0 0.10 (1)	6.25	M-E	0 / 414	0.09 (1)		
H-I	0 / 52	-78.0 -78.0 0.15 (1)	10.00	M-F	-376 / 0	0.30 (1)		
J-H	-1467 / 0	0.0 0.0 0.21 (1)	6.76	L-F	-941 / 0	0.55 (1)		
				L-G	0 / 1044	0.23 (1)		
R-Q	0 / 0	-18.5 -18.5 0.07 (4)	10.00	K-G	-405 / 0	0.24 (1)		
Q-P	0 / 1063	-18.5 -18.5 0.21 (1)	10.00	K-H	0 / 738	0.17 (1)		
P-O	0 / 787	-18.5 -18.5 0.20 (4)	10.00					
O-N	0 / 781	-18.5 -18.5 0.20 (4)	10.00					
N-M	0 / 781	-18.5 -18.5 0.20 (4)	10.00					
M-L	0 / 970	-18.5 -18.5 0.20 (1)	10.00					
L-K	0 / 552	-18.5 -18.5 0.13 (1)	10.00					
K-J	0 / 0	-18.5 -18.5 0.04 (4)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(5% OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.93")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.93")
CALCULATED VERT. DEFL.(TL) = L/999 (0.10")

CSI: TC=0.58/1.00 (A-R:1), BC=0.21/1.00 (P-Q:1), WB=0.55/1.00 (F-L:1), SSI=0.21/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

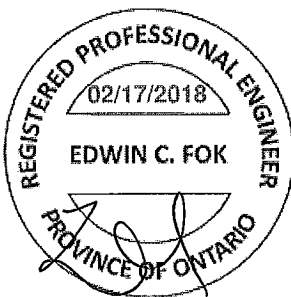
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (E) (INPUT = 0.90)
JSI METAL= 0.35 (H) (INPUT = 1.00)



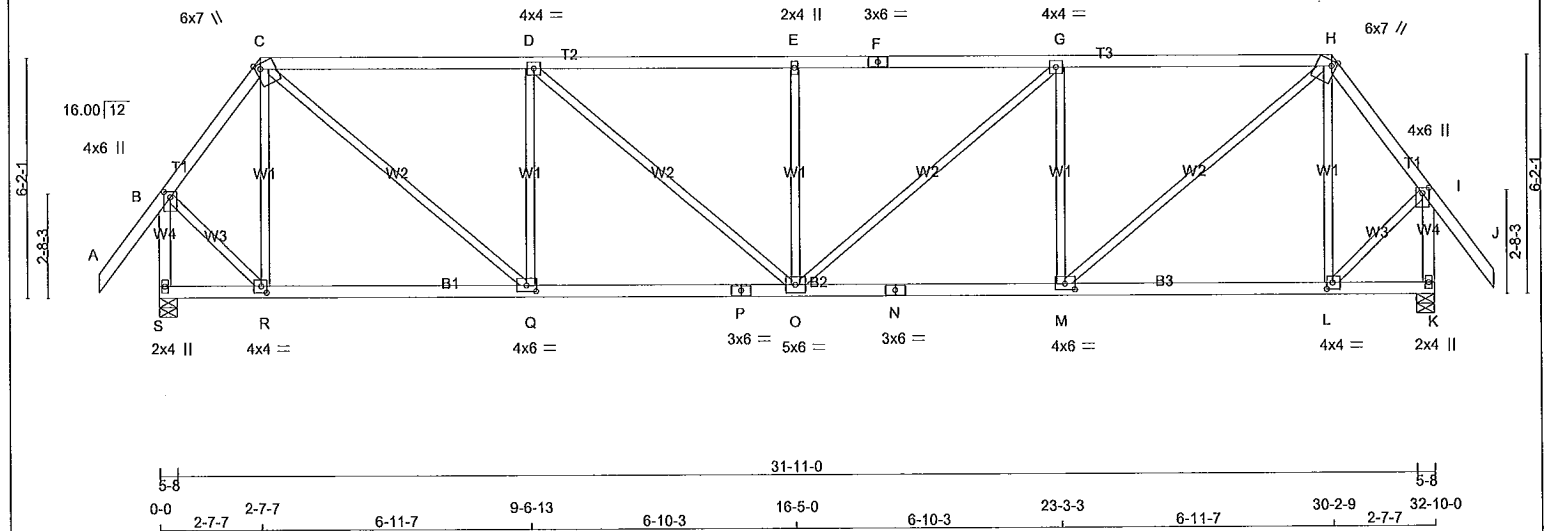
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292589	H17	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 8.200 S Jan 6 2018 MITek Industries, Inc. Sat Feb 17 12:09:07 2018 Page 1

ID:MF8IriaTok7DlbpT4B71Xys4PG-kU7x_u0CzQ95KIBcLpK_we2ar1R3sgSwjBkLszkFVA

-1-6-8 0-0 5-8 2-7-7 6-11-7 9-6-13 6-10-3 16-5-0 6-10-3 23-3-3 6-11-7 30-2-9 32-4-8 32-10-0 34-4-8
1-6-8 5-8 2-1-15 6-11-7 9-6-13 6-10-3 16-5-0 6-10-3 23-3-3 6-11-7 30-2-9 32-4-8 32-10-0 34-4-8
Scale = 1:59.4



TOTAL WEIGHT = 147 lb

LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES				
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
S - P	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	1.75	2.00
C	TTWW+m	MT20	6.0	7.0	Edge	1.50
D	TMWW-t	MT20	4.0	4.0		
E	TMW-w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW+m	MT20	6.0	7.0	Edge	1.50
I	TMVW+p	MT20	4.0	6.0	1.75	2.00
K	BMV1+p	MT20	2.0	4.0		
L	BMWW-t	MT20	4.0	4.0	2.00	1.75
N	BS-t	MT20	4.0	6.0	1.75	3.00
O	BMWW-t	MT20	3.0	6.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWW-t	MT20	3.0	6.0		
R	BMWW-t	MT20	4.0	6.0	1.75	3.00
S	BMV1+p	MT20	4.0	4.0	2.00	1.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
S	1713	0	1713	0
K	1713	0	1713	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. SNOW	MIN. LIVE	PERM. LIVE	WIND	DEAD	SOIL
S	1219	759 / 0	0 / 0	0 / 0	0 / 0	460 / 0	0 / 0
K	1219	759 / 0	0 / 0	0 / 0	0 / 0	460 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.70 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC2)	MAX. FACTORED VERT. LOAD (LC3)	MAX. FACTORED VERT. LOAD (LC4)	MAX. FACTORED VERT. LOAD (LC5)	MAX. FACTORED VERT. LOAD (LC6)	MAX. FACTORED VERT. LOAD (LC7)	MAX. FACTORED VERT. LOAD (LC8)	MAX. FACTORED VERT. LOAD (LC9)	MAX. FACTORED VERT. LOAD (LC10)
FR-TO												
A-B	0 / 52	-78.0	-78.0	0.15 (1)	10.00	R-C	-457 / 0	0.28 (1)				
B-C	-1134 / 0	-78.0	-78.0	0.11 (1)	5.85	C-Q	0 / 1615	0.36 (1)				
C-D	-1903 / 0	-78.0	-78.0	0.73 (1)	4.03	Q-D	-910 / 0	0.55 (1)				
D-E	-2262 / 0	-78.0	-78.0	0.79 (1)	3.70	D-O	0 / 473	0.11 (1)				
E-F	-2262 / 0	-78.0	-78.0	0.79 (1)	3.70	O-E	-492 / 0	0.30 (1)				
F-G	-2262 / 0	-78.0	-78.0	0.79 (1)	3.70	G-O	0 / 473	0.11 (1)				
G-H	-1903 / 0	-78.0	-78.0	0.73 (1)	4.03	M-G	-910 / 0	0.55 (1)				
H-I	-1134 / 0	-78.0	-78.0	0.11 (1)	5.85	M-H	0 / 1615	0.36 (1)				
I-J	0 / 52	-78.0	-78.0	0.15 (1)	10.00	L-H	-457 / 0	0.28 (1)				
S-B	-1710 / 0	0.0	0.0	0.25 (1)	6.36	B-R	0 / 881	0.20 (1)				
K-I	-1710 / 0	0.0	0.0	0.25 (1)	6.36	L-I	0 / 881	0.20 (1)				
S-R	0 / 0	-18.5	-18.5	0.13 (4)	10.00							
R-Q	0 / 666	-18.5	-18.5	0.24 (4)	10.00							
Q-P	0 / 1903	-18.5	-18.5	0.40 (1)	10.00							
P-O	0 / 1903	-18.5	-18.5	0.40 (1)	10.00							
O-N	0 / 1903	-18.5	-18.5	0.40 (1)	10.00							
N-M	0 / 1903	-18.5	-18.5	0.40 (1)	10.00							
M-L	0 / 666	-18.5	-18.5	0.24 (4)	10.00							
L-K	0 / 0	-18.5	-18.5	0.13 (4)	10.00							

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.09")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL) = L/360 (1.09")
CALCULATED VERT. DEFL.(TL) = L/999 (0.22")

CSI: TC=0.79/1.00 (E-G:1), BC=0.40/1.00 (O-Q:1), WB=0.55/1.00 (D-Q:1), SSI=0.26/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 L.S BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

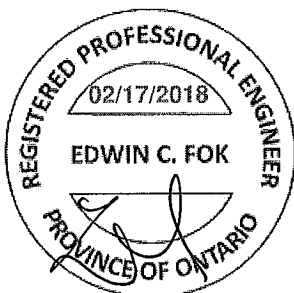
PLATE PLACEMENT TOL. = 0.250 inches

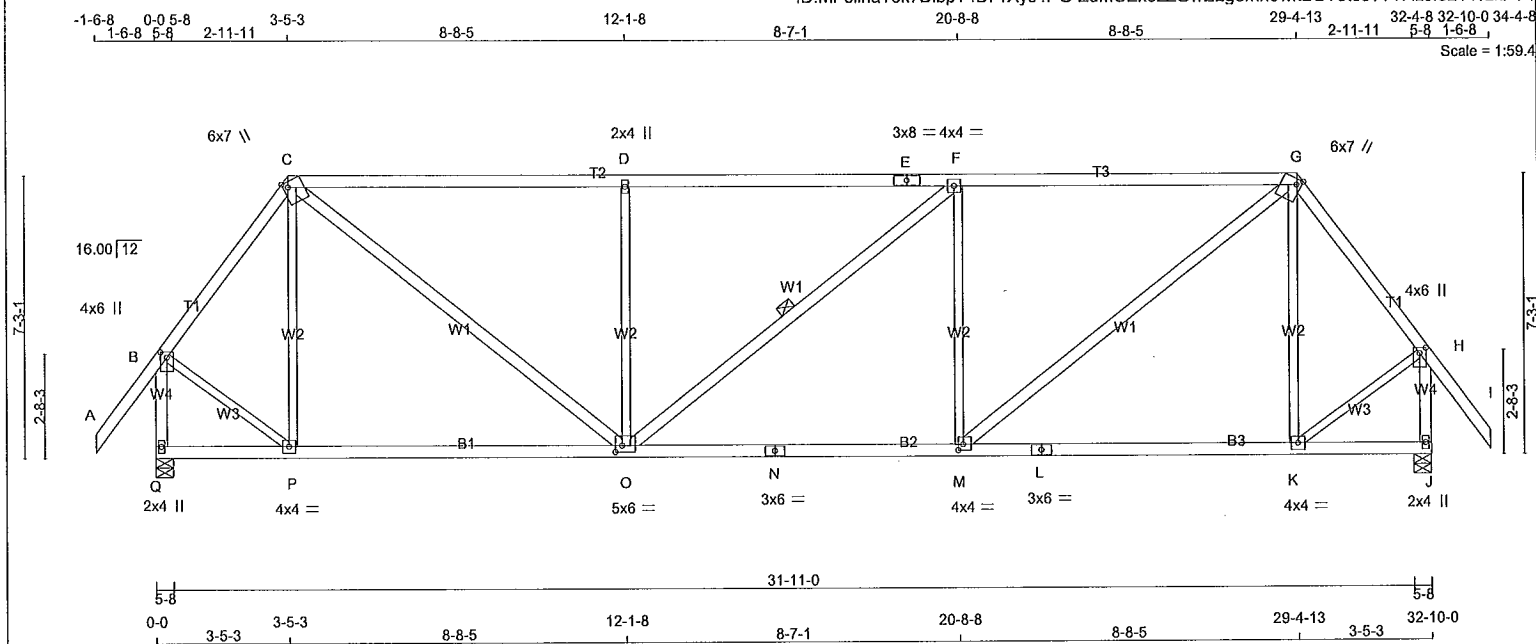
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (K) (INPUT = 0.90)
JSI METAL= 0.64 (N) (INPUT = 1.00)

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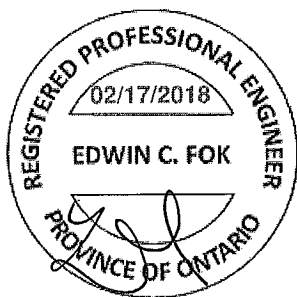
LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE		LUMBER	DESCR
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	2100F 1.8E	SPF
E - G	2x4	DRY	2100F 1.8E	SPF
G - I	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - O	2x4	DRY	No.2	SPF
O - F	2x4	DRY	No.2	SPF
M - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in Inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMWV+p	MT20	4.0	6.0	1.75 2.00
C	TTWVW+m	MT20	6.0	7.0	Edge 1.50
D	TMW+w	MT20	2.0	4.0	
E	TS-t	MT20	3.0	8.0	
F	BMWVW-t	MT20	4.0	4.0	
G	TTWVW+m	MT20	6.0	7.0	Edge 1.50
H	TMWVW+p	MT20	4.0	6.0	1.75 2.00
J	BMV1+p	MT20	2.0	4.0	
K	BMWVW-t	MT20	4.0	4.0	
L	BS-t	MT20	3.0	6.0	
M	BMWVW-t	MT20	4.0	4.0	1.75 1.50
N	BS-t	MT20	3.0	6.0	
O	BMWVWVW-t	MT20	5.0	6.0	2.00 2.00
P	BMWVW-t	MT20	4.0	4.0	
Q	BMV1+p	MT20	2.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

BEARINGS							REQD
JT	FACTORED		MAXIMUM FACTORED			INPUT	REQD
	GROSS REACTION		GROSS REACTION			BRG	BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
Q	1713	0	1713	0	0	5-8	2-9
J	1713	0	1713	0	0	5-8	2-9

UNFACTORED REACTIONS

SUN ROTATED REACTION							
1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1219	759 / 0	0 / 0	0 / 0	0 / 0	460 / 0	0 / 0
J	1219	759 / 0	0 / 0	0 / 0	0 / 0	460 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.05 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY
APPLIED

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 52	-78.0	-78.0	0.15 (1)	10.00	P-C	-334 / 0 0.31 (1)
B-C	-1213 / 0	-78.0	-78.0	0.19 (1)	5.61	C-O	0 / 1430 0.23 (1)
C-D	-1836 / 0	-78.0	-78.0	0.67 (1)	5.05	O-D	-729 / 0 0.68 (1)
D-E	-1837 / 0	-78.0	-78.0	0.66 (1)	5.05	O-F	0 / 0 0.00 (0)
E-F	-1837 / 0	-78.0	-78.0	0.66 (1)	5.05	M-F	-729 / 0 0.68 (1)
F-G	-1837 / 0	-78.0	-78.0	0.67 (1)	5.05	M-G	0 / 1431 0.23 (1)
G-H	-1212 / 0	-78.0	-78.0	0.19 (1)	5.61	K-G	-334 / 0 0.31 (1)
H-I	0 / 52	-78.0	-78.0	0.15 (1)	10.00	B-P	0 / 861 0.19 (1)
Q-B	-1705 / 0	0.0	0.0	0.25 (1)	6.36	K-H	0 / 861 0.19 (1)
J-H	-1705 / 0	0.0	0.0	0.25 (1)	6.36		
Q-P	0 / 0	-18.5	-18.5	0.20 (4)	10.00		
P-O	0 / 718	-18.5	-18.5	0.34 (4)	10.00		
O-N	0 / 1837	-18.5	-18.5	0.45 (4)	10.00		
N-M	0 / 1837	-18.5	-18.5	0.45 (4)	10.00		
M-L	0 / 717	-18.5	-18.5	0.35 (4)	10.00		
L-K	0 / 717	-18.5	-18.5	0.35 (4)	10.00		
K-J	0 / 0	-18.5	-18.5	0.20 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	21.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD		= 34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.09")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (1.09")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.67/1.00 (F-G:1) , BC=0.45/1.00 (M-O:4)
 , WB=0.68/1.00 (F-M:1) , SSI=0.32/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

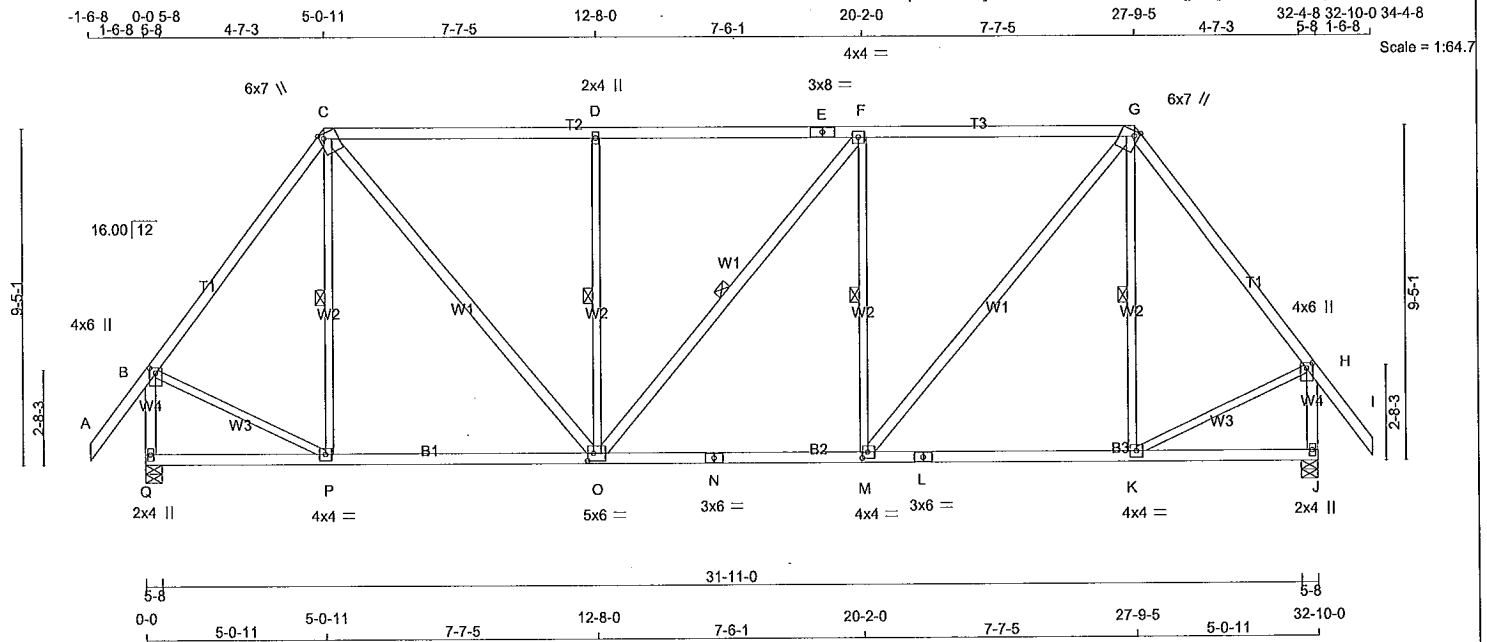
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (M) (INPUT = 0.90)
JSI METAL= 0.67 (N) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292589	H20	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 8.200 S Jan 6 2018 MITek Industries, Inc. Sat Feb 17 12:09:26 2018 Page 1
ID:MF8ilriaTok7DlbpT4B71Xys4PG-h8e6zOFVfY05g8GyEfoFxW45Xj0V6FIAIEVGzkFU



TOTAL WEIGHT = 2 X 171 = 342 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF		
C - E	2x4	DRY	No.2	SPF		
E - G	2x4	DRY	No.2	SPF		
G - I	2x4	DRY	No.2	SPF		
Q - B	2x4	DRY	No.2	SPF		
J - H	2x4	DRY	No.2	SPF		
Q - N	2x4	DRY	No.2	SPF		
N - L	2x4	DRY	No.2	SPF		
L - J	2x4	DRY	No.2	SPF		
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF		
C - O	2x4	DRY	No.2	SPF		
O - F	2x4	DRY	No.2	SPF		
M - G	2x4	DRY	No.2	SPF		

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	1.75	2.00
C	TTWW+m	MT20	6.0	7.0	Edge	1.50
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	8.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTWW+m	MT20	6.0	7.0	Edge	1.50
H	TMVW+p	MT20	4.0	6.0	1.75	2.00
J	BMV1+p	MT20	2.0	4.0		
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	4.0	2.00	1.75
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	5.0	6.0	2.50	2.00
P	BMWW-t	MT20	4.0	4.0		
Q	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	UPLIFT
Q	1713	0	1713	0
J	1713	0	1713	0

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1219	759 / 0	0 / 0	0 / 0	0 / 0	460 / 0	0 / 0
J	1219	759 / 0	0 / 0	0 / 0	0 / 0	460 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.44 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-P, D-O, F-O, F-M, G-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX.	MEMB.	MAX. FACTORED	W E B S
MEMB.	FORCE (LBS)	VERT. (PLF)	FROM	TO	CSI (LC)	UNBRAC	LENGTH	FR-TO	MEMB.
FR-TO									
A-B	0 / 52	-78.0	-78.0	0.15 (1)	10.00	P-C	-205 / 12	0.12 (1)	
B-C	-1268 / 0	-78.0	-78.0	0.47 (1)	5.22	C-O	0 / 1014	0.16 (1)	
C-D	-1406 / 0	-78.0	-78.0	0.76 (1)	4.46	O-D	-637 / 0	0.37 (1)	
D-E	-1406 / 0	-78.0	-78.0	0.76 (1)	4.44	O-F	-1 / 0	0.00 (1)	
E-F	-1406 / 0	-78.0	-78.0	0.76 (1)	4.44	M-F	-638 / 0	0.37 (1)	
F-G	-1407 / 0	-78.0	-78.0	0.77 (1)	4.44	M-G	0 / 1016	0.16 (1)	
G-H	-1268 / 0	-78.0	-78.0	0.47 (1)	5.22	K-G	-206 / 12	0.12 (1)	
H-I	0 / 52	-78.0	-78.0	0.15 (1)	10.00	B-P	0 / 829	0.19 (1)	
Q-B	-1680 / 0	0.0	0.0	0.24 (1)	6.41	K-H	0 / 829	0.19 (1)	
J-H	-1680 / 0	0.0	0.0	0.24 (1)	6.41				
Q-P	0 / 0	-18.5	-18.5	0.18 (4)	10.00				
P-O	0 / 755	-18.5	-18.5	0.28 (4)	10.00				
O-N	0 / 1407	-18.5	-18.5	0.34 (1)	10.00				
N-M	0 / 1407	-18.5	-18.5	0.34 (1)	10.00				
M-L	0 / 755	-18.5	-18.5	0.28 (4)	10.00				
L-K	0 / 755	-18.5	-18.5	0.28 (4)	10.00				
K-J	0 / 0	-18.5	-18.5	0.18 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(65 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.09")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (1.09")
CALCULATED VERT. DEFL.(TL) = L/999 (0.13")

CSI: TC=0.77/1.00 (F-G:1), BC=0.34/1.00 (M-O:1), WB=0.37/1.00 (F-M:1), SSI=0.28/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

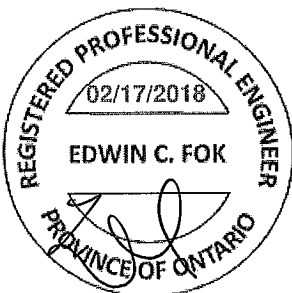
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

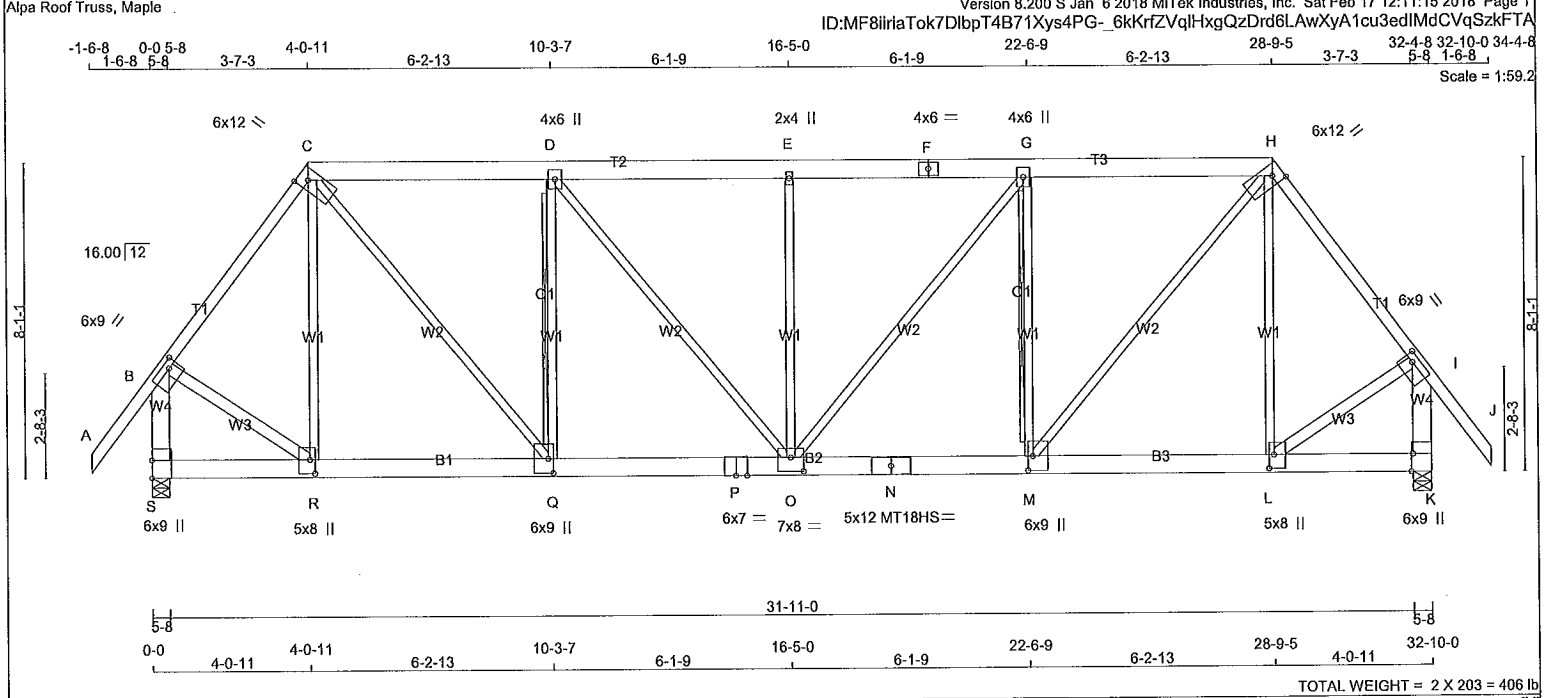
JSI GRIP= 0.89 (M) (INPUT = 0.90)
JSI METAL= 0.52 (N) (INPUT = 1.00)



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CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292589	H21	1	2	TRUSS DESC.	



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - F	2x6	DRY	No.2	SPF	
F - H	2x6	DRY	No.2	SPF	
H - J	2x4	DRY	No.2	SPF	
S - B	2x6	DRY	No.2	SPF	
K - I	2x6	DRY	No.2	SPF	
S - P	2x6	DRY	1650F 1.5E	SPF	
P - N	2x6	DRY	1650F 1.5E	SPF	
N - K	2x6	DRY	1650F 1.5E	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	
B - R	2x4	DRY	No.2	SPF	
L - I	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.
 DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	12	TOP
H - J	12	TOP
C - F	12	TOP
F - H	12	TOP
S - B	12	TOP
K - I	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
S - P	9	SIDE(282.4)
P - N	9	SIDE(282.4)
N - K	9	SIDE(282.4)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	
2x4	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.
 TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
S	10986	0	10986	0
K	10986	0	10986	0

1ST LCASE	MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
S	7830	4794	0	0	0	3036
K	7830	4794	0	0	0	3036

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K
 BEARING SIZE FACTOR = 1.15 AT JNT(S) S, K (BASED ON SUPPORT DEPTH = 1-8)

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.02 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.
 2x4 DRY SPF No.2 T-BRACE AT D-Q, G-M

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
 TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)
MEMB.									
FR-TO									
A-B	0 / 52	-78.0	-78.0	0.08 (1)	10.00	R-C	0 / 541	0.07 (1)	
B-C	-8820 / 0	-78.0	-78.0	0.66 (1)	3.02	Q-D	0 / 7917	0.98 (1)	
C-D	-10306 / 0	-78.0	-78.0	0.32 (1)	3.64	Q-E	-2487 / 0	0.69 (1)	
D-E	-11898 / 0	-78.0	-78.0	0.36 (1)	3.38	D-O	0 / 2543	0.31 (1)	
E-F	-11898 / 0	-78.0	-78.0	0.36 (1)	3.38	O-E	-459 / 0	0.26 (1)	
F-G	-11898 / 0	-78.0	-78.0	0.36 (1)	3.38	O-G	0 / 2543	0.31 (1)	
G-H	-10306 / 0	-78.0	-78.0	0.32 (1)	3.64	M-G	-2487 / 0	0.69 (1)	
H-I	-8820 / 0	-78.0	-78.0	0.66 (1)	3.02	M-H	0 / 7917	0.98 (1)	
I-J	0 / 52	-78.0	-78.0	0.08 (1)	10.00	L-H	0 / 541	0.07 (1)	
S-B	-10155 / 0	0.0	0.0	0.51 (1)	4.73	B-R	0 / 5987	0.53 (1)	
K-I	-10155 / 0	0.0	0.0	0.51 (1)	4.73	L-I	0 / 5987	0.53 (1)	
S-R	0 / 0	-583.4	-583.4	0.40 (1)	10.00				
R-Q	0 / 5295	-583.4	-583.4	0.67 (1)	10.00				
Q-P	0 / 10306	-583.4	-583.4	0.88 (1)	10.00				
P-O	0 / 10306	-583.4	-583.4	0.88 (1)	10.00				
O-N	0 / 10306	-583.4	-583.4	0.88 (1)	10.00				
N-M	0 / 10306	-583.4	-583.4	0.88 (1)	10.00				
M-L	0 / 5295	-583.4	-583.4	0.67 (1)	10.00				
L-K	0 / 0	-583.4	-583.4	0.40 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 21.0 PSF
 DL = 6.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CStdGirder
 START DISTANCE = 0-0
 START SPAN CARRIED = 25-5-1
 END DISTANCE = 32-10-0
 END SPAN CARRIED = 25-5-1
 END WALL WIDTH = 0-0
 APPLIED TO FRONT SIDE OF BOTTOM CHORD.
 - ADDTL LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, OBC 2018
 - CSA 086-09, CSA 086-14
 - TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.09")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.20")
 ALLOWABLE DEFL.(TL) = L/360 (1.09")
 CALCULATED VERT. DEFL.(TL) = L/998 (0.39")

CSI: TC=0.66/1.00 (B-C:1), BC=0.88/1.00 (O-Q:1)
 , WB=0.98/1.00 (Q-Q:1), SSI=0.60/1.00 (M-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

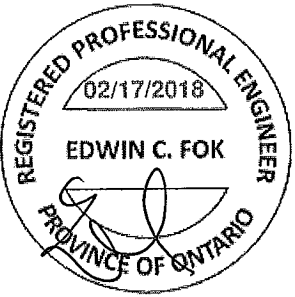
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 788 1987 1656
 MT18HS 511 354 2455 1382 3004 2010

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (M) (INPUT = 0.90)
 JSI METAL= 0.95 (L) (INPUT = 1.00)



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292589	H21	1	2	TRUSS DESC.	

Alpa Roof Truss, Maple

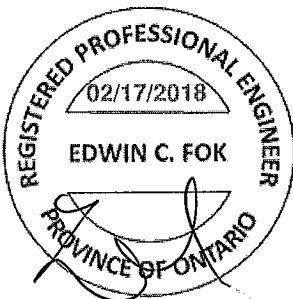
Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Sat Feb 17 12:11:15 2018 Page 2
ID:MF8IiriaTok7DlbpT4B71Xys4PG- 6kKrfZVqlHxgQzDrd6LAWXyA1cu3edIMdCVqSzkFTA

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	6.0	9.0	2.00	2.75
C	TTWW+h	MT20	6.0	12.0	3.25	2.75
D	TMWW+t	MT20	4.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	4.0	6.0		
G	TMWW+t	MT20	4.0	6.0		
H	TTWW+h	MT20	6.0	12.0	3.25	2.75
I	TMVW-t	MT20	6.0	9.0	2.00	2.75
K	BMV1+t	MT20	6.0	9.0	Edge	0.50
L	BMWW+t	MT20	5.0	8.0	4.25	1.50
M	BMWW+t	MT20	6.0	9.0	4.50	1.50
N	BS-t	MT18HS	5.0	12.0		
O	BMWWW-t	MT20	7.0	8.0	4.25	4.00
P	BS-t	MT20	6.0	7.0		
Q	BMWW+t	MT20	6.0	9.0	4.50	1.50
R	BMWW+t	MT20	5.0	8.0	4.25	1.50
S	BMV1+t	MT20	6.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4
for Part 9 design as per OBC 9.23.13.11,
and no less than 2x4 for Part 4 design

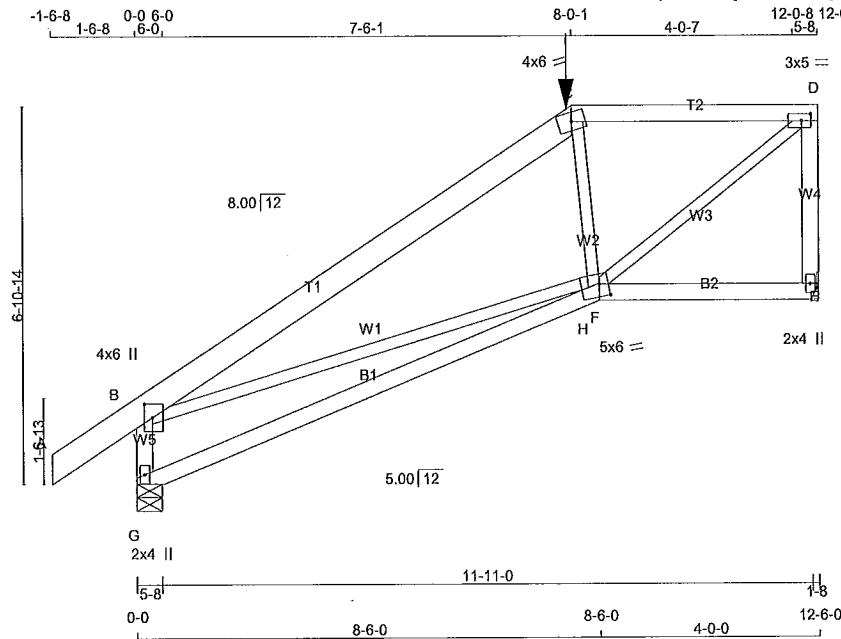


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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292589	H22T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 8.200 S Jan 6 2018 MITok Industries, Inc. Sat Feb 17 12:20:58 2018 Page 1
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TOTAL WEIGHT = 59 lb

LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES				
A - C	2x6	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	3.00	1.75
C	TTW-m	MT20	4.0	6.0		
D	TMVW-t	MT20	3.0	5.0	1.50	2.00
E	BMV1+p	MT20	2.0	4.0		
F	BBWW-m	MT20	5.0	6.0	2.75	2.00
G	BMV1+p	MT20	2.0	4.0		

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 173.4 lbs FACTORED DOWN AT 8-0-1 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
E	832	0	832	0	0/0	0/0
G	847	0	847	0	0/0	0/0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM.LIVE	WIND			
E	592	367/0	0/0	0/0	0/0	224/0	0/0
G	603	376/0	0/0	0/0	0/0	227/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS			
MAX. FACTORED		FACTORED			MAX. FACTORED			
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO			FR-TO		
A- B	0 / 36	-78.0	-78.0	0.08 (1)	10.00	C- F	-233 / 19	0.05 (1)
B- C	-879 / 0	-78.0	-78.0	0.49 (1)	6.25	F- D	0 / 896	0.22 (1)
C- D	-693 / 0	-100.7	-100.7	0.40 (1)	6.25	B- F	0 / 752	0.19 (1)
E- D	-784 / 0	0.0	0.0	0.16 (1)	7.81			
G- B	-746 / 0	0.0	0.0	0.09 (1)	7.81			
G- H	0 / 0	-23.9	-23.9	0.56 (4)	10.00			
H- F	0 / 0	-23.9	-23.9	0.56 (4)	10.00			
F- E	0 / 0	-23.9	-23.9	0.12 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	8-0-1	-173	-173		FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHlp
LEFT SETBACK = 8-0-1
RIGHT SETBACK = 0-0
END SETBACK = 3-2-0
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADD'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= 1/360 (0.42")
CALCULATED VERT. DEFL.(LL)= 1/999 (0.02")
ALLOWABLE DEFL.(TL)= 1/360 (0.42")
CALCULATED VERT. DEFL.(TL)= 1/433 (0.35")

CSI: TC=0.49/1.00 (B-C:1), BC=0.56/1.00 (F-G:4), WB=0.22/1.00 (D-F:1), SSI=0.19/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

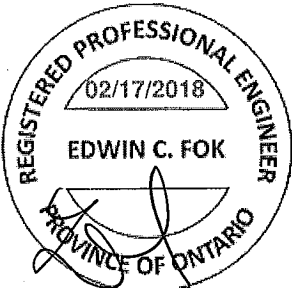
PLATE PLACEMENT TOL. = 0.250 inches

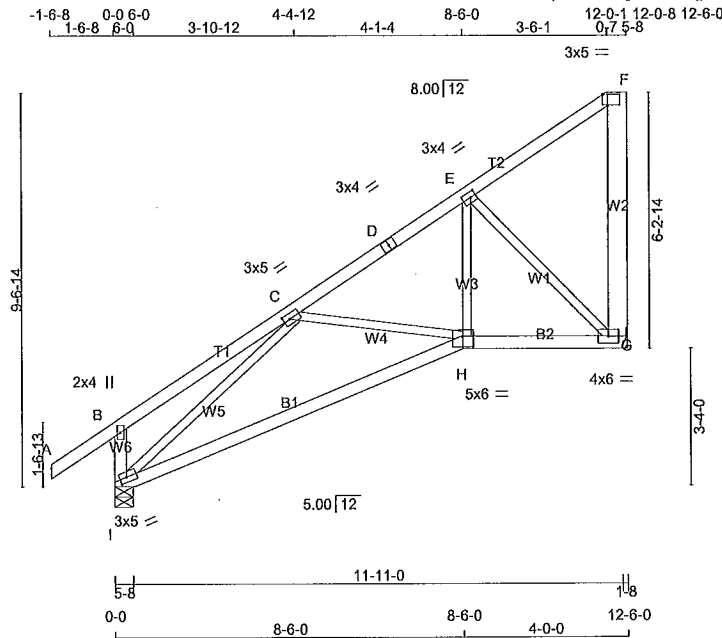
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (D) (INPUT = 0.90)
JSI METAL= 0.52 (B) (INPUT = 1.00)

A-18023453

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design





Scale = 1:56.1

TOTAL WEIGHT = 61 lb

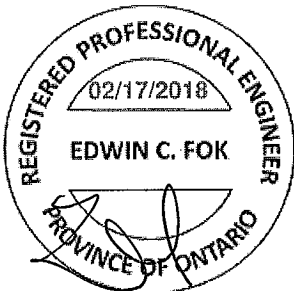
LUMBER				
N. L. G. A. RULES				
CHORDS		SIZE	LUMBER	DESCR
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x6	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY; SEASONED LUMBER.

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWW-t	MT20	3.0	5.0	1.50	1.75
D	TS-t	MT20	3.0	4.0		
E	TMWW-t	MT20	3.0	4.0	1.50	1.50
F	TVM-p	MT20	3.0	5.0	Edge	1.50
G	BMVW-t	MT20	4.0	6.0		
H	BBWW-t	MT20	5.0	6.0	3.00	3.00
I	BMVW-t	MT20	3.0	5.0	1.50	2.00

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			FACTORED			MAXIMUM FACTORED			INPUT		REQRD	
GROSS REACTION			GROSS REACTION			GROSS REACTION			BRG		BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX	IN-SX	IN-SX	IN-SX
G	603	0	603	0	0	HANGER BY OTHERS						
MIN. SEAT SIZE: 1-8												
I	729	0	729	0	0	5-8		1-8				

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
G	430	262 / 0	0 / 0	0 / 0	0 / 0	168 / 0	0 / 0
I	517	330 / 0	0 / 0	0 / 0	0 / 0	187 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LO. CSI (L.C)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (L.C)
FR-TO		FROM TO			FR-TO		
A-B	0 / 35	-78.0	-78.0 0.15 (1)	10.00	H-E	0 / 423	0.10 (1)
B-C	0 / 17	-78.0	-78.0 0.18 (1)	10.00	E-G	-654 / 0	0.28 (1)
C-D	-589 / 0	-78.0	-78.0 0.14 (1)	6.25	I-C	-935 / 0	0.59 (1)
D-E	-589 / 0	-78.0	-78.0 0.14 (1)	6.25	C-H	-175 / 0	0.06 (1)
E-F	-18 / 0	-78.0	-78.0 0.13 (1)	6.25			
G-F	-120 / 0	0.0	0.0 0.07 (1)	7.81			
I-B	-264 / 0	0.0	0.0 0.03 (1)	7.81			
I-H	0 / 716	-18.5	-18.5 0.44 (4)	10.00			
H-G	0 / 477	-18.5	-18.5 0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:				
TOP	CH.	LL =	21.0	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.42")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.42")
CALCULATED VERT. DEFL.(TL) = L/ 555 (0.27")

CSI: TC=0.18/1.00 (B-C:1), BC=0.44/1.00 (H-I:4),
WB=0.59/1.00 (C-I:1), SSI=0.13/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES						
PLATE	GRIP (DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

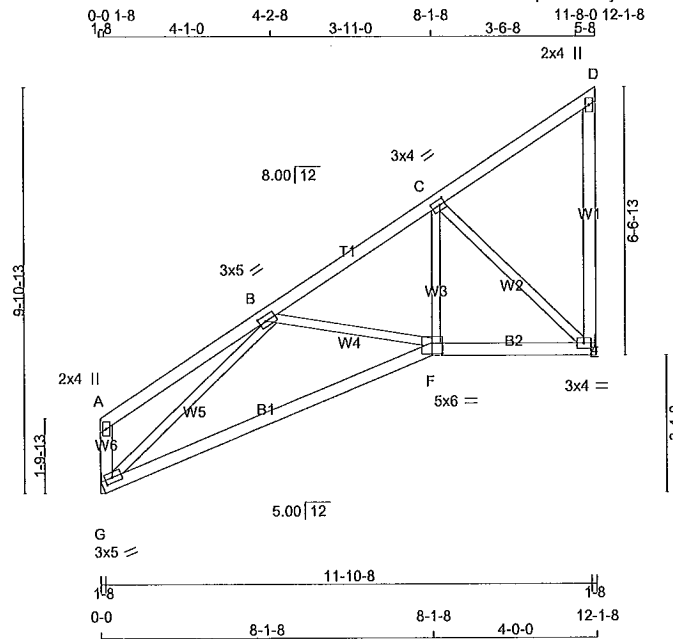
JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.29 (I) (INPUT = 1.00)

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Alpa Roof Truss, Maple

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Sat Feb 17 12:18:41 2018 Page 1
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Scale = 1:56.3



TOTAL WEIGHT = 5 X 54 = 272 lb

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0		
B	TMWW-I	MT20	3.0	5.0	1.50	2.00
C	TMWW-I	MT20	3.0	4.0	1.50	1.50
D	TMV+p	MT20	2.0	4.0		
E	BMVW-I	MT20	3.0	4.0		
F	BBWW-I	MT20	5.0	6.0	3.00	3.00
G	BMVW-I	MT20	3.0	5.0	1.50	2.25

BEARINGS

UNFACTORED REACTIONS

BRACING

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX FACTORED CSI (LC)
FR-TO		FROM TO			FR-TO		
G-A	-132 / 0	0.0	0.0	0.01 (1)	7.81	F-C	0 / 396
A-B	0 / 116	-78.0	-78.0	0.16 (1)	10.00	C-E	-626 / 0
B-C	-562 / 0	-78.0	-78.0	0.13 (1)	6.25	G-B	-874 / 0
C-D	-17 / 0	-78.0	-78.0	0.13 (1)	6.25	B-F	-141 / 0
E-D	-121 / 0	0.0	0.0	0.10 (1)	7.81		
G-F	0 / 656	-18.5	-18.5	0.41 (4)	10.00		
F-E	0 / 457	-18.5	-18.5	0.13 (4)	10.00		

SPECIFIED LOADS:			
TOP CH.	LL =	21.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD		= 34.4	PSF

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= $L/360$ (0.40")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.01")
ALLOWABLE DEFL.(TL)= $L/360$ (0.40")
CALCULATED VERT. DEFL.(TL) = $L/641$ (0.23")

CSI: TC=0.16/1.00 (A-B:1), BC=0.41/1.00 (F-G:4),
WB=0.51/1.00 (B-G:1), SSI=0.12/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

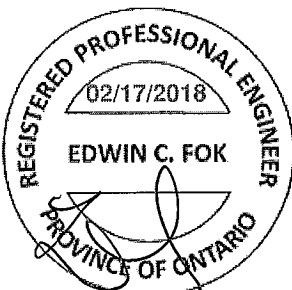
PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

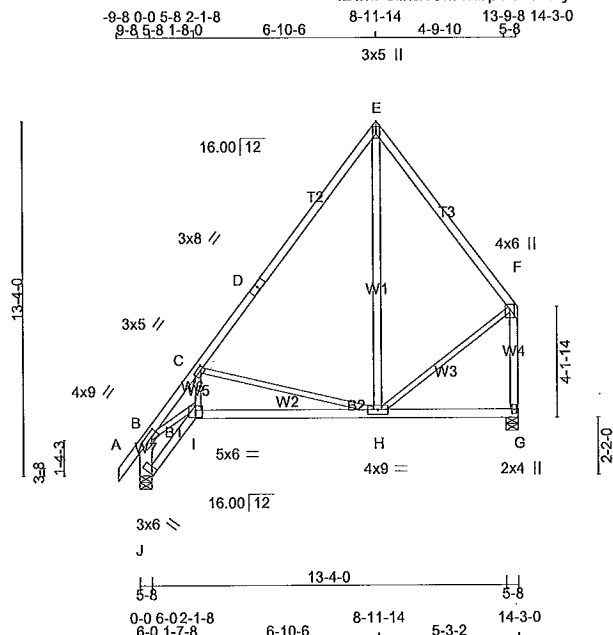
JSI GRIP= 0.87 (G) (INPUT = 0.90)
JSI METAL= 0.26 (G) (INPUT = 1.00)

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



A-18023455

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292589	H25T	7	1	TRUSS DESC.	



LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
J - B	2x6	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
H - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMVW-I	MT20	4.0	9.0 2.00 3.00
C	TMWW-I	MT20	3.0	5.0 1.50 1.50
D	TS-I	MT20	3.0	8.0
E	TTW+p	MT20	3.0	5.0 2.00 Edge
F	TMVW+p	MT20	4.0	6.0 2.00 2.00
G	BMV1+p	MT20	2.0	4.0
H	BMVWW-I	MT20	4.0	9.0
I	BBWW-I	MT20	5.0	6.0 3.00 3.00
J	BVM1+I	MT20	3.0	6.0 Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
JT	758	0	758	0	5-8	1-8
J						
G	687	0	687	0	5-8	1-8

UNFACTORED REACTIONS							
1ST LCASE	MAX/MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	539	337 / 0	0 / 0	0 / 0	0 / 0	202 / 0	0 / 0
G	490	299 / 0	0 / 0	0 / 0	0 / 0	191 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G
BEARING SIZE FACTOR = 1.15 AT JNT(S) J (BASED ON SUPPORT DEPTH = 1-8)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED L.C1 MAX (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED MAX (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-78.0 -78.0	0.05 (1)	I-C	0 / 656	0.15 (1)	
B-C	-1514 / 0	-78.0 -78.0	0.37 (1)	C-H	-743 / 0	0.69 (1)	
C-D	-404 / 0	-78.0 -78.0	0.46 (1)	H-E	0 / 165	0.04 (4)	
D-E	-404 / 0	-78.0 -78.0	0.46 (1)	H-F	0 / 270	0.06 (1)	
E-F	-366 / 0	-78.0 -78.0	0.28 (1)	B-I	0 / 1136	0.26 (1)	
J-B	-739 / 0	0.0 0.0	0.05 (1)				
G-F	-655 / 0	0.0 0.0	0.19 (1)				
J-I	0 / 0	-18.5 -18.5	0.02 (4)	10.00			
I-H	0 / 938	-18.5 -18.5	0.29 (4)	10.00			
H-G	0 / 0	-18.5 -18.5	0.21 (4)	10.00			

TOTAL WEIGHT = 7 X 79 = 554 lb [M/F]

DESIGN CRITERIA		
SPECIFIED LOADS:	TOP CH.	LL = 21.0 PSF
		DL = 6.0 PSF
	BOT CH.	LL = 0.0 PSF
		DL = 7.4 PSF
	TOTAL LOAD	= 34.4 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.48")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.48")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.46/1.00 (C-E:1), BC=0.29/1.00 (H-I:4), WB=0.69/1.00 (C-H:1), SSI=0.15/1.00 (C-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

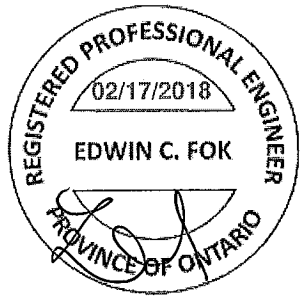
NAIL VALUES					
PLATE	GRIP(DRY)	SHEAR	SECTION		
	(PSI)	(PLI)	(PLI)		
	MAX	MIN	MAX	MIN	MAX
MT20	618	354	1667	788	1987

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (I) (INPUT = 0.90)
JSI METAL= 0.47 (I) (INPUT = 1.00)

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292589	H26	1	1	TRUSS DESC.	

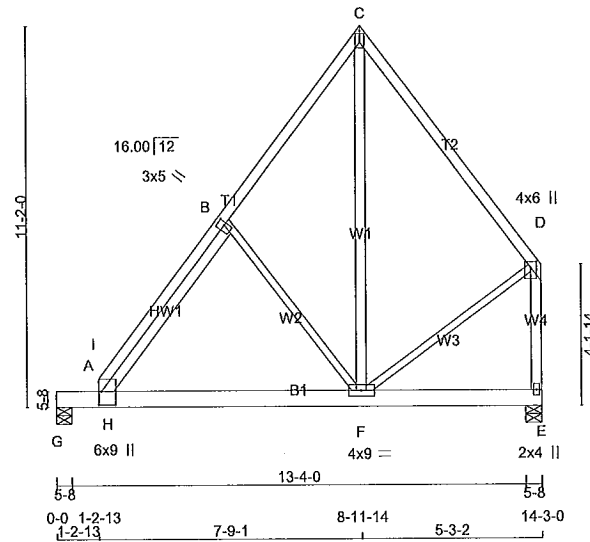
Alpa Roof Truss, Maple

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Sat Feb 17 12:20:28 2018 Page 1

ID:MF8ilriaTok7DlbpT4B71Xys4PG-IUeNvIGNbfyTWPIVKGX14XxWqyR27x7hWAFgkuzkFKX

0-0 5-8 1-2-13 5-0-7 8-11-14 13-9-8 14-3-0
 5-8 9-5 3-9-11 3-11-7 4-9-10 5-8
 3x5 II

Scale = 1:67.6



TOTAL WEIGHT = 84 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	
G - E	2x6	DRY	No.2	SPF	
REINFORCING MEMBERS					
HW1	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT					
F - D	2x3	DRY	No.2	SPF	
B - F	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
A	TBMW+H	MT20	6.0	9.0	4.75	1.00
B	TMWW+H	MT20	3.0	5.0	2.25	1.00
C	TTW+p	MT20	3.0	5.0	2.00	Edge
D	TBMW+p	MT20	4.0	6.0	2.00	2.00
E	BMV1+p	MT20	2.0	4.0		
F	BMVWW-t	MT20	4.0	9.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED GROSS REACTION	FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
G	687	0	687	0
E	687	0	687	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
G	490	299 / 0	0 / 0	191 / 0	0 / 0
E	490	299 / 0	0 / 0	191 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.35 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC2)	MAX. FACTORED VERT. LOAD (LC3)	MAX. FACTORED VERT. LOAD (LC4)	MAX. FACTORED VERT. LOAD (LC5)	MAX. FACTORED VERT. LOAD (LC6)	MAX. FACTORED VERT. LOAD (LC7)	MAX. FACTORED VERT. LOAD (LC8)	MAX. FACTORED VERT. LOAD (LC9)	MAX. FACTORED VERT. LOAD (LC10)
FR-TO													
A - I	-1427 / 0	-78.0	-78.0	0.11 (1)	5.35	F - C	0 / 292	0.05 (1)					
I - B	-1298 / 0	-78.0	-78.0	0.26 (1)	5.38	F - D	0 / 280	0.06 (1)					
B - C	-414 / 0	-78.0	-78.0	0.22 (1)	6.25	B - F	-298 / 0	0.24 (1)					
C - D	-382 / 0	-78.0	-78.0	0.39 (1)	6.25	H - I	0 / 56	0.00 (1)					
E - D	-672 / 0	0.0	0.0	0.18 (1)	7.81	H - B	0 / 741	0.12 (1)					
G - A	0 / 0	-96.5	-96.5	0.41 (1)	10.00								
A - H	0 / 809	-18.5	-18.5	0.51 (1)	10.00								
H - F	0 / 405	-18.5	-18.5	0.24 (1)	10.00								
F - E	0 / 0	-18.5	-18.5	0.13 (4)	10.00								

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 21.0 PSF
 DL = 6.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, OBC 2018
 - CSA 086-09, CSA 086-14
 - TPIC 2011, TPIC 2014

(55% OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.48")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
 ALLOWABLE DEFL.(TL)= L/360 (0.48")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.13")

CSI: TC=0.39/1.00 (C-D:1), BC=0.51/1.00 (A-H:1), WB=0.24/1.00 (B-F:1), SSI=0.41/1.00 (A-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

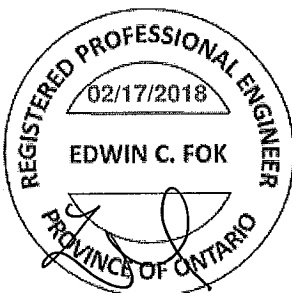
NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (B) (INPUT = 0.90)
 JSI METAL= 0.38 (B) (INPUT = 1.00)

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



A-18023457

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292589	H27	3	1	TRUSS DESC.	

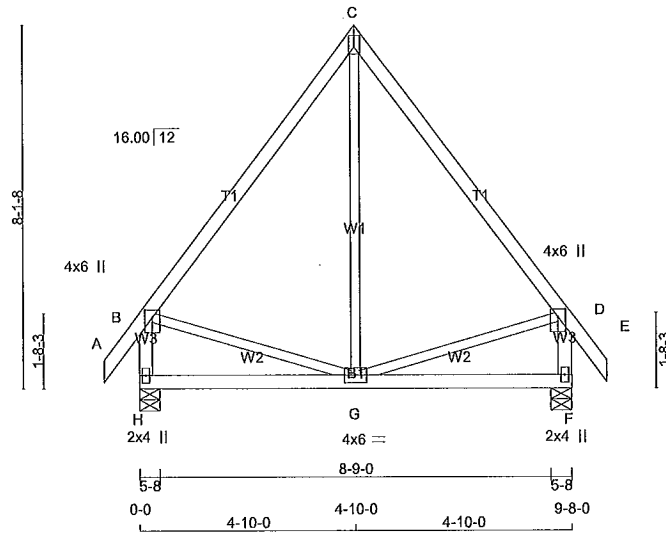
Alpa Roof Truss, Maple

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Sat Feb 17 12:21:04 2018 Page 1

ID:MF8iiriaTok7DlbpT4B71Xys4PG-FQp_T3hy7JVMrxVQrqATY7QJfTpTZGB_omNA?pzKFJz

9-8 0-0 5-8 4-4-8 4-10-0 4-4-8 9-2-8 9-8-0 10-5-8
 9-8 5-8 4-4-8 4-10-0 4-4-8 8-8 9-8
 3x5 II

Scale = 1:51.6



TOTAL WEIGHT = 3 X 50 = 149 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF		
C - E	2x4	DRY	No.2	SPF		
H - B	2x4	DRY	No.2	SPF		
F - D	2x4	DRY	No.2	SPF		
H - F	2x4	DRY	No.2	SPF		
ALL WEBS	2x3	DRY	No.2	SPF		
EXCEPT						

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TTW+p	MT20	3.0	5.0	2.00	Edge
D	TMVW+p	MT20	4.0	6.0	2.00	2.00
F	BMV1+p	MT20	2.0	4.0		
G	BMVWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
H	537	0	537	0	5-8	1-8
F	537	0	537	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX / MIN	COMPONENT REACTIONS	WIND	DEAD	SOIL
H	381	241 / 0	0 / 0	0 / 0	140 / 0	0 / 0
F	381	241 / 0	0 / 0	0 / 0	140 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 28	-78.0	-78.0 0.05 (1)	10.00	G-C	0 / 90	0.03 (4)
B-C	-254 / 0	-78.0	-78.0 0.23 (1)	6.25	B-G	0 / 158	0.04 (1)
C-D	-254 / 0	-78.0	-78.0 0.23 (1)	6.25	G-D	0 / 158	0.04 (1)
D-E	0 / 28	-78.0	-78.0 0.05 (1)	10.00			
H-B	-503 / 0	0.0	0.0 0.05 (1)	7.81			
F-D	-503 / 0	0.0	0.0 0.05 (1)	7.81			
H-G	0 / 0	-18.5	-18.5 0.12 (4)	10.00			
G-F	0 / 0	-18.5	-18.5 0.12 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 21.0 PSF
 DL = 6.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, OBC 2018
 - CSA 088-09, CSA 088-14
 - TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.32")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
 ALLOWABLE DEFL.(TL) = L/360 (0.32")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.23/1.00 (B-C:1), BC=0.12/1.00 (G-H:4), WB=0.04/1.00 (D-G:1), SSI=0.09/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

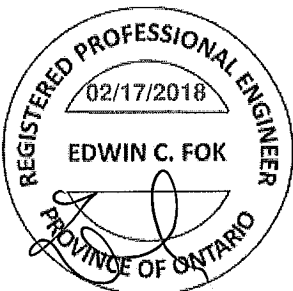
NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

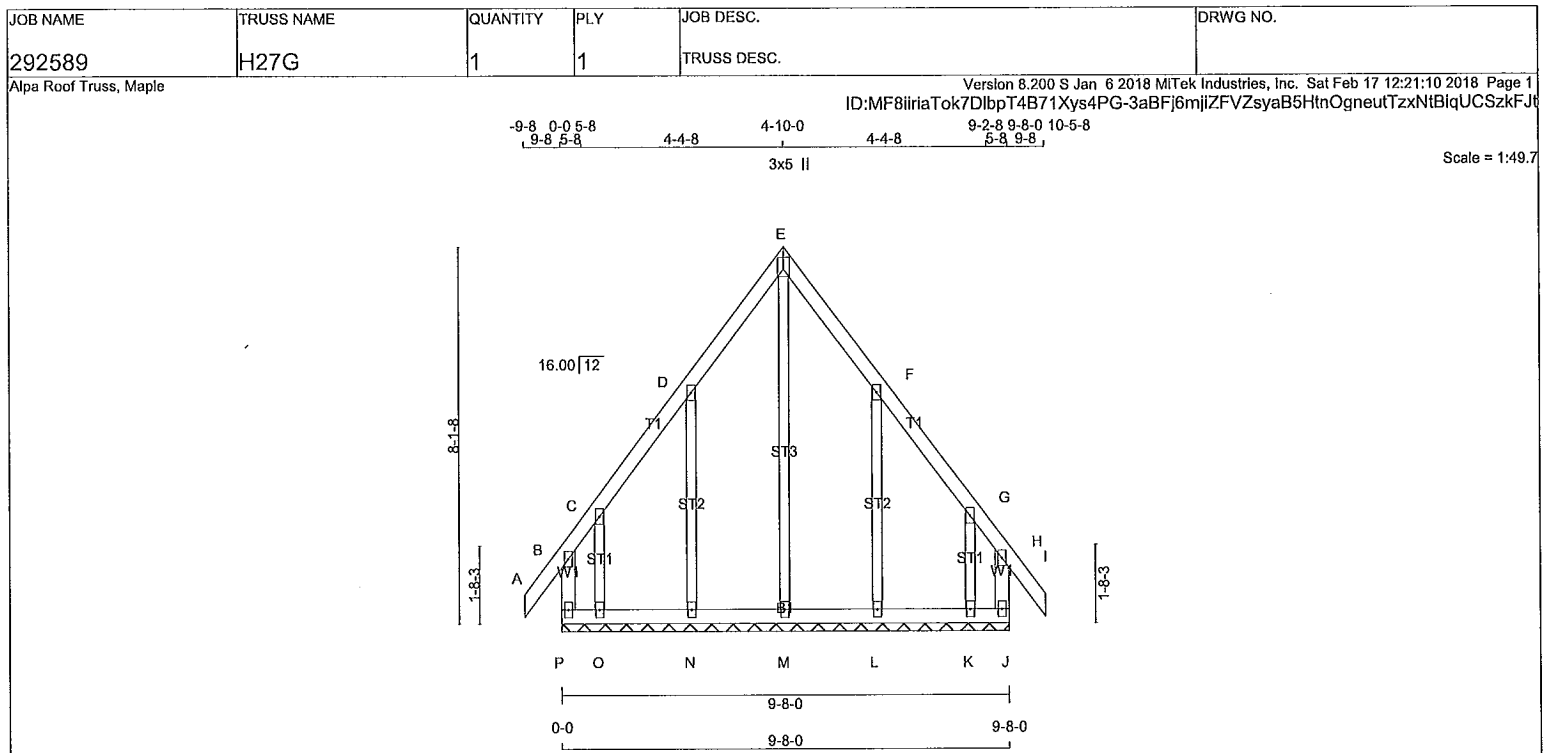
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.29 (B) (INPUT = 0.90)
 JSI METAL= 0.14 (D) (INPUT = 1.00)

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



A-18023458



TOTAL WEIGHT = 54 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
P - B	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				
GABLE STUDS SPACED AT 2-0-0 OC.				

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	2.0	4.0
C, D, F, G				
C	TMW+w	MT20	2.0	4.0
E	TTW+p	MT20	3.0	5.0
H	TMV+p	MT20	2.0	4.0
J	BMV1+p	MT20	2.0	4.0
K, L, M, N, O				
K	BMW1+w	MT20	2.0	4.0
P	BMV1+p	MT20	2.0	4.0
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.				

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX CSI (LC)	UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED MAX CSI (LC)		
FR-TO		FROM TO			FR-TO				
P-B	-136 / 0	0.0	0.0	0.02 (1)	7.81	M-E	-154 / 0	0.18 (1)	
A-B	0 / 28	-78.0	-78.0	0.05 (1)	10.00	N-D	-164 / 0	0.07 (1)	
B-C	-36 / 0	-78.0	-78.0	0.04 (1)	6.25	O-C	-71 / 0	0.01 (1)	
C-D	-6 / 0	-78.0	-78.0	0.04 (1)	10.00	L-F	-164 / 0	0.07 (1)	
D-E	-7 / 0	-78.0	-78.0	0.04 (1)	10.00	K-G	-71 / 0	0.01 (1)	
E-F	-7 / 0	-78.0	-78.0	0.04 (1)	10.00				
F-G	-6 / 0	-78.0	-78.0	0.04 (1)	10.00				
G-H	-36 / 0	-78.0	-78.0	0.04 (1)	6.25				
H-I	0 / 28	-78.0	-78.0	0.05 (1)	10.00				
J-H	-136 / 0	0.0	0.0	0.02 (1)	7.81				
P-O	0 / 11	-18.5	-18.5	0.02 (1)	10.00				
O-N	0 / 7	-18.5	-18.5	0.02 (4)	10.00				
N-M	0 / 3	-18.5	-18.5	0.02 (4)	10.00				
M-L	0 / 3	-18.5	-18.5	0.02 (4)	10.00				
L-K	0 / 7	-18.5	-18.5	0.02 (4)	10.00				
K-J	0 / 11	-18.5	-18.5	0.02 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	34.4	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.05/1.00 (H-I:1), BC=0.02/1.00 (J-K:1), WB=0.18/1.00 (E-M:1), SSI=0.04/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

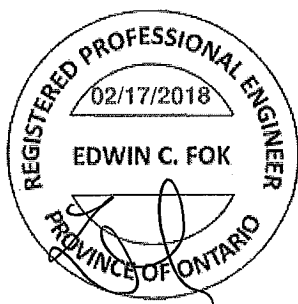
PLATE	GRIP (DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 788	1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

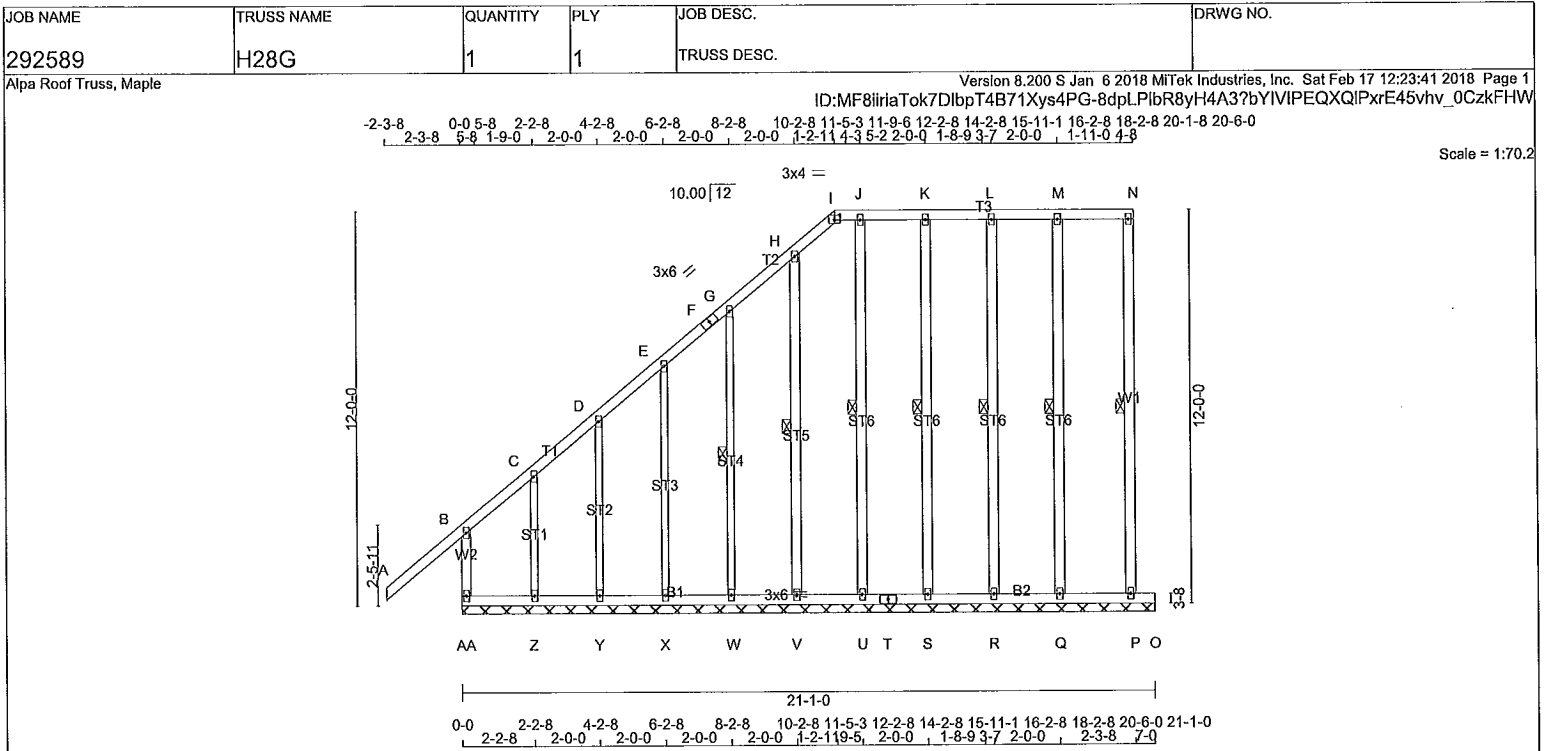
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (E) (INPUT = 0.90)
JSI METAL= 0.08 (F) (INPUT = 1.00)

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



A-18023459



LUMBER		N. L. G. A. RULES		LUMBER		DESCR.	
CHORDS	SIZE						
A - F	2x4	DRY	No.2	SPF			
F - I	2x4	DRY	No.2	SPF			
P - N	2x4	DRY	No.2	SPF			
AA - B	2x4	DRY	No.2	SPF			
I - N	2x4	DRY	No.2	SPF			
AA - T	2x4	DRY	No.2	SPF			
T - O	2x4	DRY	No.2	SPF			
ALL WEBS EXCEPT		2x4	DRY	No.2	SPF		
W - G	2x3	DRY	No.2	SPF			
X - E	2x3	DRY	No.2	SPF			
Y - D	2x3	DRY	No.2	SPF			
Z - C	2x3	DRY	No.2	SPF			
ALL GABLE WEBS EXCEPT		2x4	DRY	No.2	SPF		
ST6	2x3	DRY	No.2	SPF			
ST7	2x3	DRY	No.2	SPF			
ST8	2x3	DRY	No.2	SPF			
ST9	2x3	DRY	No.2	SPF			
DRY: SEASONED LUMBER.							
GABLE STUDS SPACED AT 2'-0" OC.							

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
B TMV+p	MT20	2.0	4.0	
C, D, E, G, H, J, K, L, M				
C TMW+w	MT20	2.0	4.0	
F TS-t	MT20	3.0	6.0	
I TT-I	MT20	3.0	4.0	1.75 2.00
N TMV+p	MT20	2.0	4.0	
P BMV1+p	MT20	2.0	4.0	
Q, R, S, U, V, W, X, Y, Z				
Q BMW1+w	MT20	2.0	4.0	
T BS-t	MT20	3.0	6.0	
AA BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

PROVIDE ANCHORAGE AT BEARING JOINT Q FOR 150 LBS FACTORED UPLIFT BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S).

BRACING
FOR SECTION I-N, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF N-P, M-Q, L-R, K-S, J-U, H-V, G-W.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0/60	-78.0 -78.0	0.31 (1)	10.00	Q-M	-204/0	0.16 (1)
B-C	-72/0	-78.0 -78.0	0.25 (1)	6.25	R-L	-165/0	0.13 (1)
C-D	0/4	-78.0 -78.0	0.05 (1)	10.00	S-K	-171/0	0.13 (1)
D-E	-9/0	-78.0 -78.0	0.05 (1)	10.00	U-J	-173/0	0.14 (1)
E-F	-4/0	-78.0 -78.0	0.03 (1)	10.00	V-H	-161/0	0.10 (1)
F-G	-4/0	-78.0 -78.0	0.03 (1)	10.00	W-G	-152/0	0.09 (1)
G-H	0/0	-78.0 -78.0	0.04 (1)	10.00	X-E	-150/0	0.16 (1)
H-I	-20/0	-78.0 -78.0	0.04 (1)	6.25	Y-D	-181/0	0.10 (1)
P-N	-83/0	0.0 0.0	0.01 (1)	6.25	Z-C	-42/0	0.01 (1)
AA-B	-370/0	0.0 0.0	0.08 (1)	7.81			
I-J	0/1	-85.5 -85.5	0.04 (1)	2.00			
J-K	0/0	-85.5 -85.5	0.04 (1)	2.00			
K-L	0/0	-85.5 -85.5	0.04 (1)	2.00			
L-M	0/0	-85.5 -85.5	0.05 (1)	2.00			
M-N	0/0	-85.5 -85.5	0.05 (1)	2.00			
AA-Z	0/9	-18.5 -18.5	0.08 (1)	10.00			
Z-Y	0/8	-18.5 -18.5	0.02 (4)	10.00			
Y-X	0/5	-18.5 -18.5	0.02 (4)	10.00			
X-W	0/3	-18.5 -18.5	0.01 (4)	10.00			
W-V	0/2	-18.5 -18.5	0.01 (4)	10.00			
V-U	0/0	-18.5 -18.5	0.01 (4)	10.00			
U-T	0/0	-18.5 -18.5	0.01 (4)	10.00			
T-S	0/0	-18.5 -18.5	0.01 (4)	10.00			
S-R	0/0	-18.5 -18.5	0.01 (4)	10.00			
R-Q	0/0	-18.5 -18.5	0.02 (4)	10.00			
Q-P	0/0	-18.5 -18.5	0.02 (4)	10.00			
P-O	0/0	-18.5 -18.5	0.02 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.I. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.31/1.00 (A-B:1), BC=0.08/1.00 (Z-AA:1), WB=0.16/1.00 (E-X:1), SSI=0.11/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

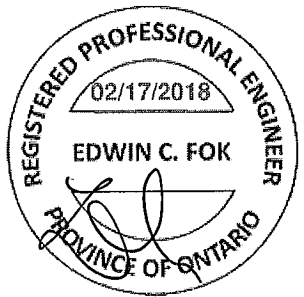
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

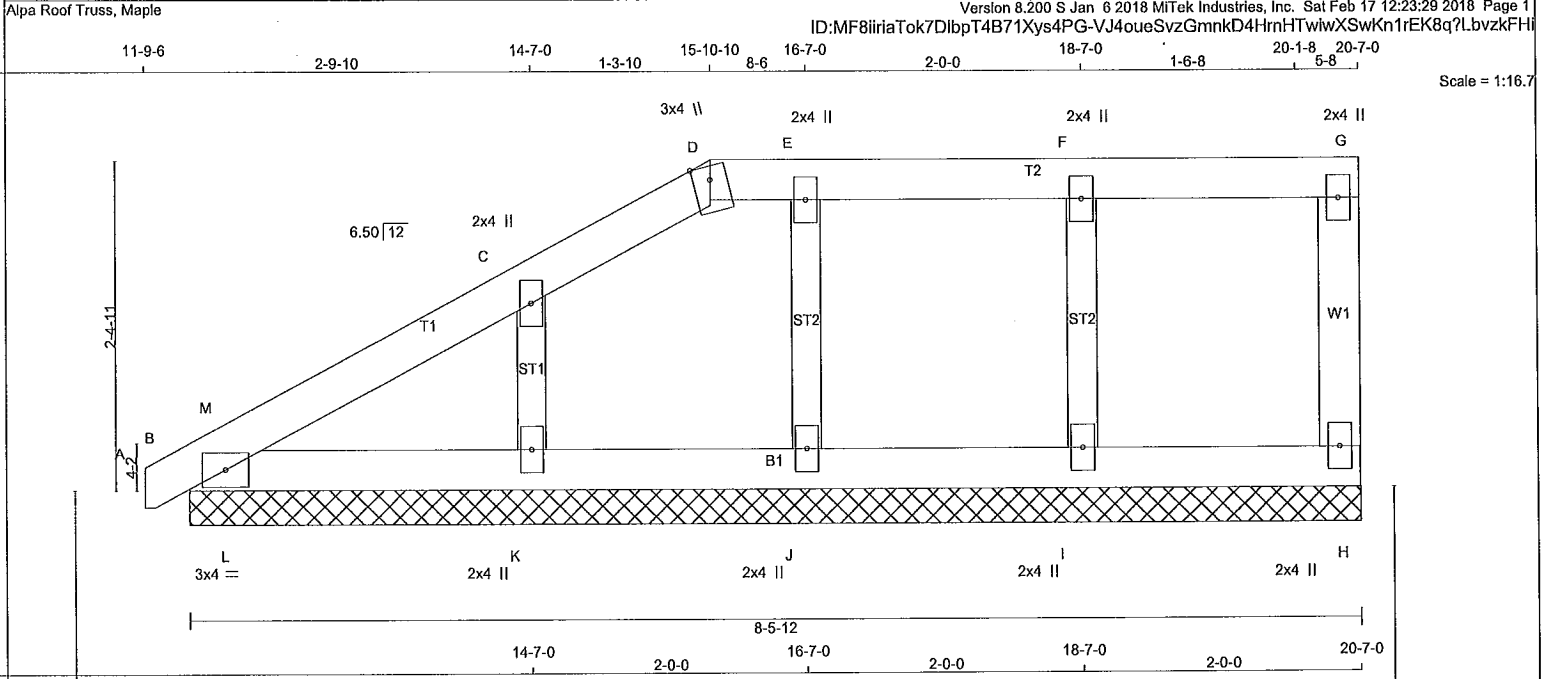
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.64 (I) (INPUT = 0.90)
JSI METAL= 0.20 (B) (INPUT = 1.00)

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292589	H29P	1	1	TRUSS DESC.	



LUMBER				
N. L. G. A. RULES	SIZE	LUMBER	DESCR.	
CHORDS				
A - D	2x4 DRY	No.2	SPF	
D - G	2x4 DRY	No.2	SPF	
H - G	2x4 DRY	No.2	SPF	
B - H	2x4 DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3 DRY	No.2	SPF	
ALL GABLE WEBS	2x3 DRY	No.2	SPF	
DRY: SEASONED LUMBER.				
GABLE STUDS SPACED AT 2-0-0 OC.				

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
B TMB-I	MT20	3.0	4.0	
C, E, F				
C TMW+w	MT20	2.0	4.0	
D TT+m	MT20	3.0	4.0	Edge
G TMV+p	MT20	2.0	4.0	
H BMV1+p	MT20	2.0	4.0	
I, J, K				
I BMW1+w	MT20	2.0	4.0	
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.				

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING				
TOTAL LOAD CASES: (4)				
CHORDS		WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MAX. FACTORED FORCE (LBS)
FR-TO		FROM	TO	
A-B	0 / 6	-78.0	-78.0 0.01 (1)	10.00
B-M	-18 / 0	-78.0	-78.0 0.01 (4)	6.25
M-C	0 / 1	-78.0	-78.0 0.06 (1)	10.00
C-D	-14 / 0	-78.0	-78.0 0.06 (1)	6.25
D-E	0 / 0	-78.0	-78.0 0.05 (1)	10.00
E-F	0 / 0	-78.0	-78.0 0.05 (1)	10.00
F-G	0 / 0	-78.0	-78.0 0.04 (1)	10.00
H-G	-63 / 0	0.0	0.0 0.01 (1)	7.81
B-L	0 / 9	-18.5	-18.5 0.03 (1)	10.00
L-K	0 / 9	-18.5	-18.5 0.03 (1)	10.00
K-J	0 / 0	-18.5	-18.5 0.03 (4)	10.00
J-I	0 / 0	-18.5	-18.5 0.02 (4)	10.00
I-H	0 / 0	-18.5	-18.5 0.02 (4)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 21.0 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, OBC 2018

- CSA 086-09, CSA 086-14

- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.06/1.00 (C-D:1), BC=0.03/1.00 (B-L:1), WB=0.03/1.00 (F-I:1), SSI=0.07/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

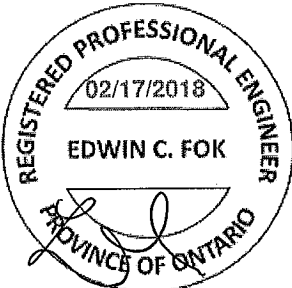
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
MT20	618	354	1667
	788	1987	1656
PLATE PLACEMENT TOL. = 0.250 Inches			
PLATE ROTATION TOL. = 5.0 Deg.			
JSI GRIP= 0.11 (C) (INPUT = 0.90)			
JSI METAL= 0.08 (C) (INPUT = 1.00)			

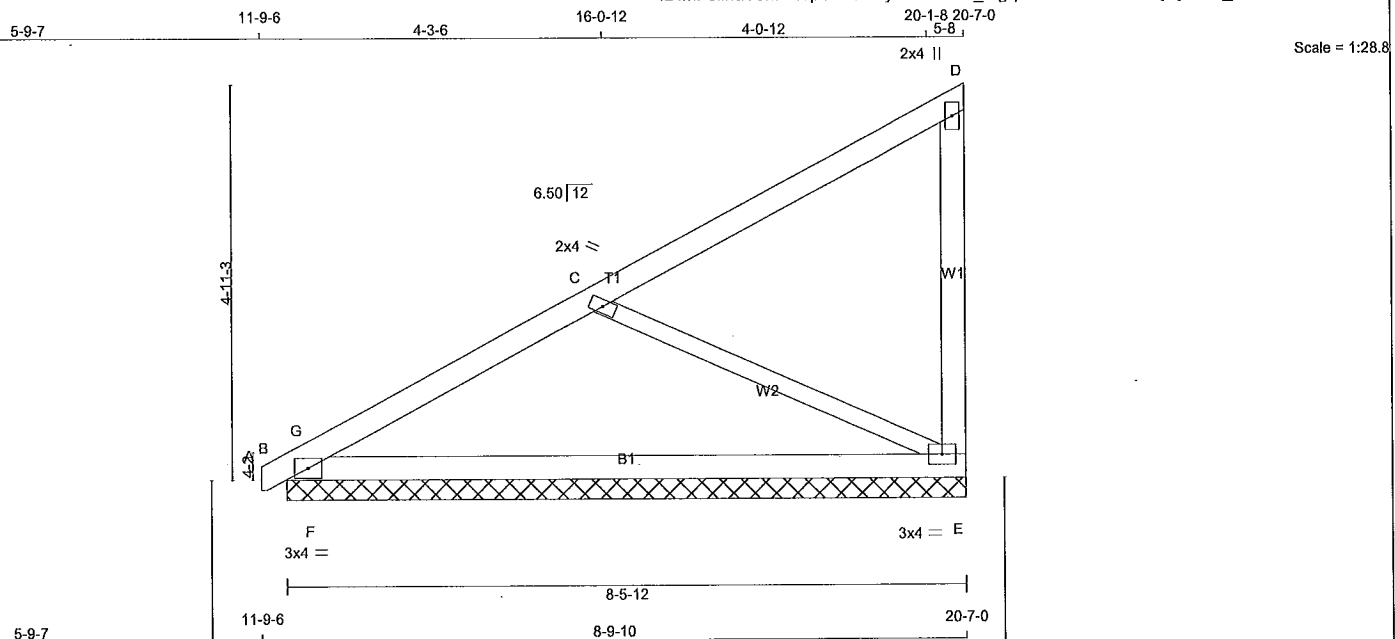
NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292589	H30P	3	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 8.200 S Jan 6 2018 MITek Industries, Inc. Sat Feb 17 12:23:48 2018 Page 1
ID:MF8iiriaTok7DlbpT4B71Xys4PG-Rzk_t8gqV695W81xSH7xBjDjMaIC_OA7WH6smlzkFHP



TOTAL WEIGHT = 3 X 30 = 90 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2	
E - D	2x4	DRY	No.2	
B - E	2x4	DRY	No.2	
ALL WEBS	2x3	DRY	No.2	
DRY: SEASONED LUMBER.				

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0			12-1-8
C	TMV+w	MT20	2.0	4.0			
D	TMV+p	MT20	2.0	4.0			
E	BMVW1-I	MT20	3.0	4.0			

DESCR.
SPF
SPF
SPF
SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED			MAXIMUM FACTORED			INPUT	REQRD
GROSS REACTION			GROSS REACTION			BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	409	0	409	0	0	8-5-12 (8-5-12)	15
R	438	0	438	0	0	8-5-12 (8-5-12)	15

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	292	178 / 0	0 / 0	0 / 0	0 / 0	114 / 0	0 / 0
B	312	194 / 0	0 / 0	0 / 0	0 / 0	118 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MAX. FACTORED		FACTORED		MAX. FACTORED		FACTORED		
MEMB.	FORCE	VERT. LOAD	LC1 MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PLF)	CSI (LC)	UNBRAC		(LBS)	CSI (LC)	
FR-TO		FROM	TO	LENGTH	FR-TO			
A- B	0 / 6	-78.0	-78.0	0.01 (1)	10.00	C- E	-487 / 0	0.20 (1)
B- G	-676 / 0	-78.0	-78.0	0.27 (4)	6.25	F- G	0 / 359	0.00 (1)
G- C	-460 / 0	-78.0	-78.0	0.27 (4)	6.25			
C- D	-20 / 0	-78.0	-78.0	0.26 (1)	6.25			
E- D	-138 / 0	0.0	0.0	0.05 (1)	7.81			
B- F	0 / 441	-18.5	-18.5	0.24 (4)	10.00			
F- F	0 / 441	-18.5	-18.5	0.29 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.27/1.00 (B-G:4), BC=0.29/1.00 (E-F:4), WB=0.20/1.00 (C-E:1), SSI=0.33/1.00 (B-G:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

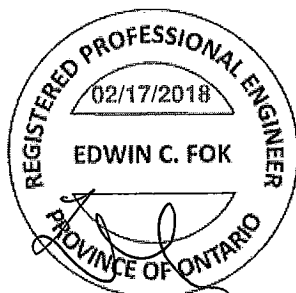
NAIL VALUES	PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	MAX MIN	
MT20	618	354	1667	788
			1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

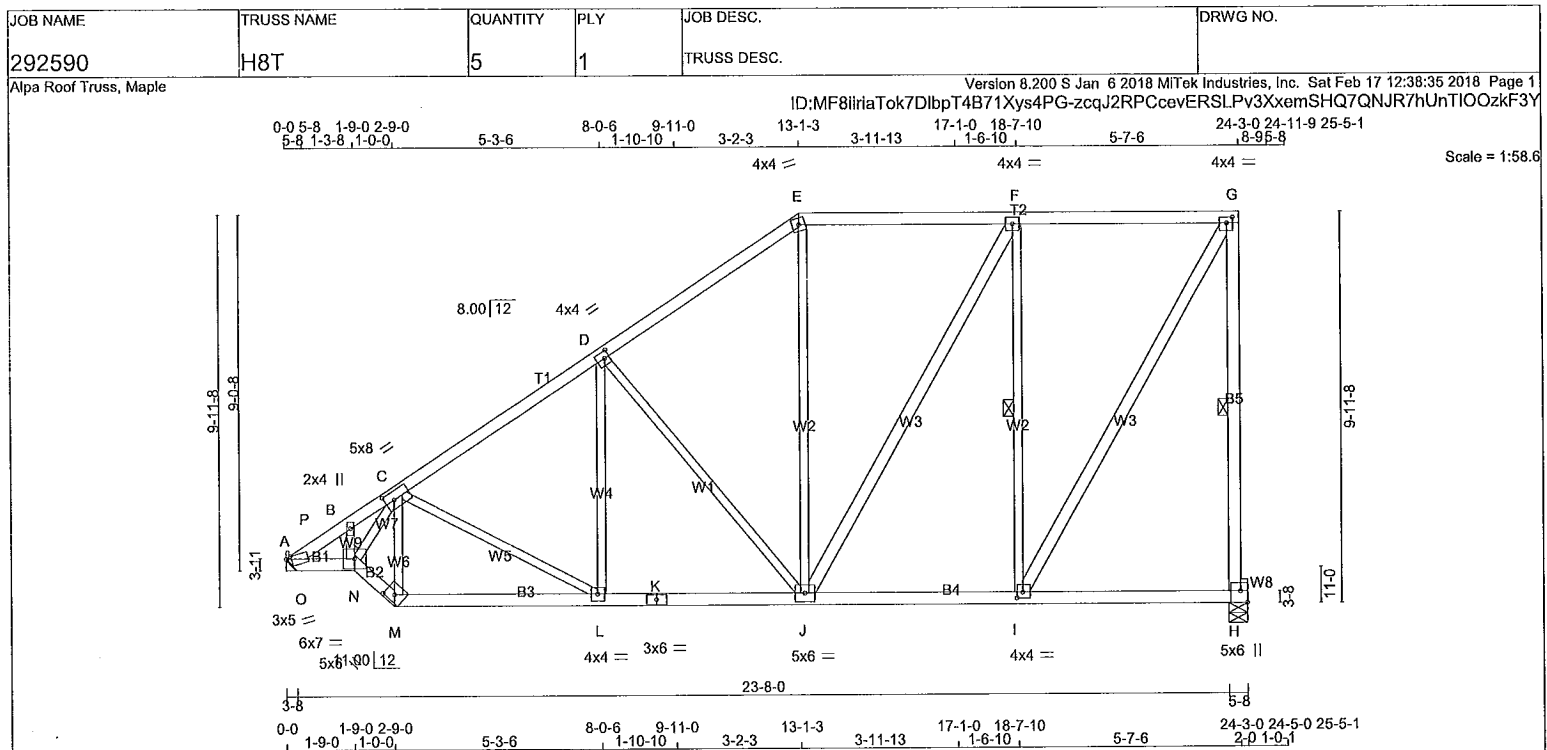
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.67 (E) (INPUT = 0.90)
JSI METAL= 0.24 (C) (INPUT = 1.00)

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



A-18023463



LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
A - N	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
J - F	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TBM1-m	MT20	3.0	5.0	0.50 1.75
B	TMW+w	MT20	2.0	4.0	
C	TMWWW-t	MT20	5.0	8.0	2.50 2.75
D	TMWW-t	MT20	4.0	4.0	2.00 1.50
E	TTW-m	MT20	4.0	4.0	
F	TMWW-t	MT20	4.0	4.0	
G	TMVV-t	MT20	4.0	4.0	1.75 1.75
H	BMVW1+p	MT20	5.0	6.0	3.50 Edge
I	BMWW-t	MT20	4.0	4.0	1.75 1.75
J	BMWWW-t	MT20	5.0	6.0	
K	BS-t	MT20	3.0	6.0	
L	BMWW-t	MT20	4.0	4.0	
M	BBW-t	MT20	5.0	6.0	2.00 3.00
N	BBWW-t	MT20	6.0	7.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
A	1181	0	1181	0	0	IN-SX
H	1226	0	1226	0	0	HANGER BY OTHERS

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
A	844	506 / 0	0 / 0	0 / 0	0 / 0	338 / 0	0 / 0
H	880	506 / 0	0 / 0	0 / 0	0 / 0	374 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

FOR SECTION E-G, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.68 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-H, F-I.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-P	-1890 / 0	-78.0 -78.0	0.04 (1)	4.68	N-B	0 / 165	0.04 (1)
P-B	-1973 / 0	-78.0 -78.0	0.16 (1)	4.68	N-C	0 / 1073	0.24 (1)
B-C	-1854 / 0	-78.0 -78.0	0.17 (1)	4.79	M-C	-936 / 0	0.17 (1)
C-D	-1266 / 0	-78.0 -78.0	0.26 (1)	5.45	J-E	0 / 162	0.04 (4)
D-E	-920 / 0	-78.0 -78.0	0.25 (1)	6.14	C-L	-82 / 0	0.04 (1)
E-F	-746 / 0	-85.5 -85.5	0.34 (1)	2.00	L-D	0 / 132	0.04 (4)
F-G	-571 / 0	-85.5 -85.5	0.33 (1)	2.00	D-J	-497 / 0	0.69 (1)
					J-F	0 / 358	0.06 (1)
A-O	0 / 1598	-18.5 -18.5	0.34 (1)	10.00	I-F	-894 / 0	0.60 (1)
O-N	0 / 1599	-18.5 -18.5	0.34 (1)	10.00	I-G	0 / 1160	0.19 (1)
N-M	0 / 1466	-18.5 -18.5	0.24 (1)	10.00	O-P	-211 / 0	0.00 (1)
M-L	0 / 1116	-18.5 -18.5	0.24 (1)	10.00			
L-K	0 / 1081	-18.5 -18.5	0.23 (1)	10.00			
K-J	0 / 1061	-18.5 -18.5	0.23 (1)	10.00			
J-I	0 / 571	-18.5 -18.5	0.19 (4)	10.00			
I-H	0 / 0	-18.5 -18.5	0.14 (4)	10.00			
H-G	-1186 / 0	0.0 0.0	0.57 (1)	5.87			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 21.0	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.80")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.80")
CALCULATED VERT. DEFL.(TL) = L/999 (0.09")

CSI: TC=0.34/1.00 (E-F:1), BC=0.57/1.00 (G-H:1), WB=0.69/1.00 (D-J:1), SI=0.23/1.00 (E-F:1)

DOL LUMBER=1.00 MAX=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

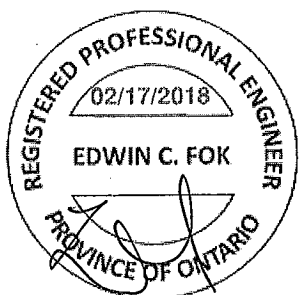
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	618	354	1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

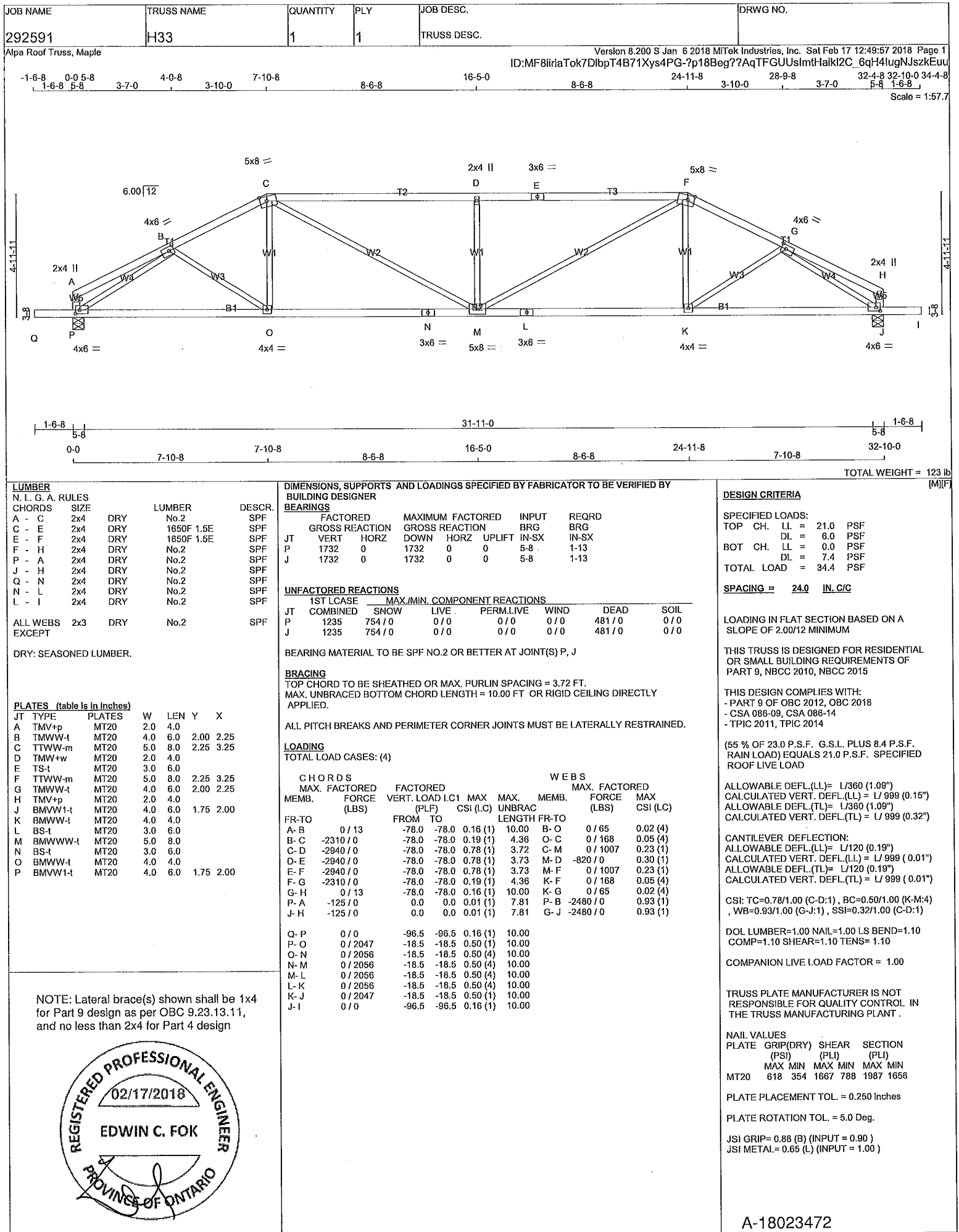
JSI GRIP= 0.90 (A) (INPUT = 0.90)
JSI METAL= 0.54 (A) (INPUT = 1.00)

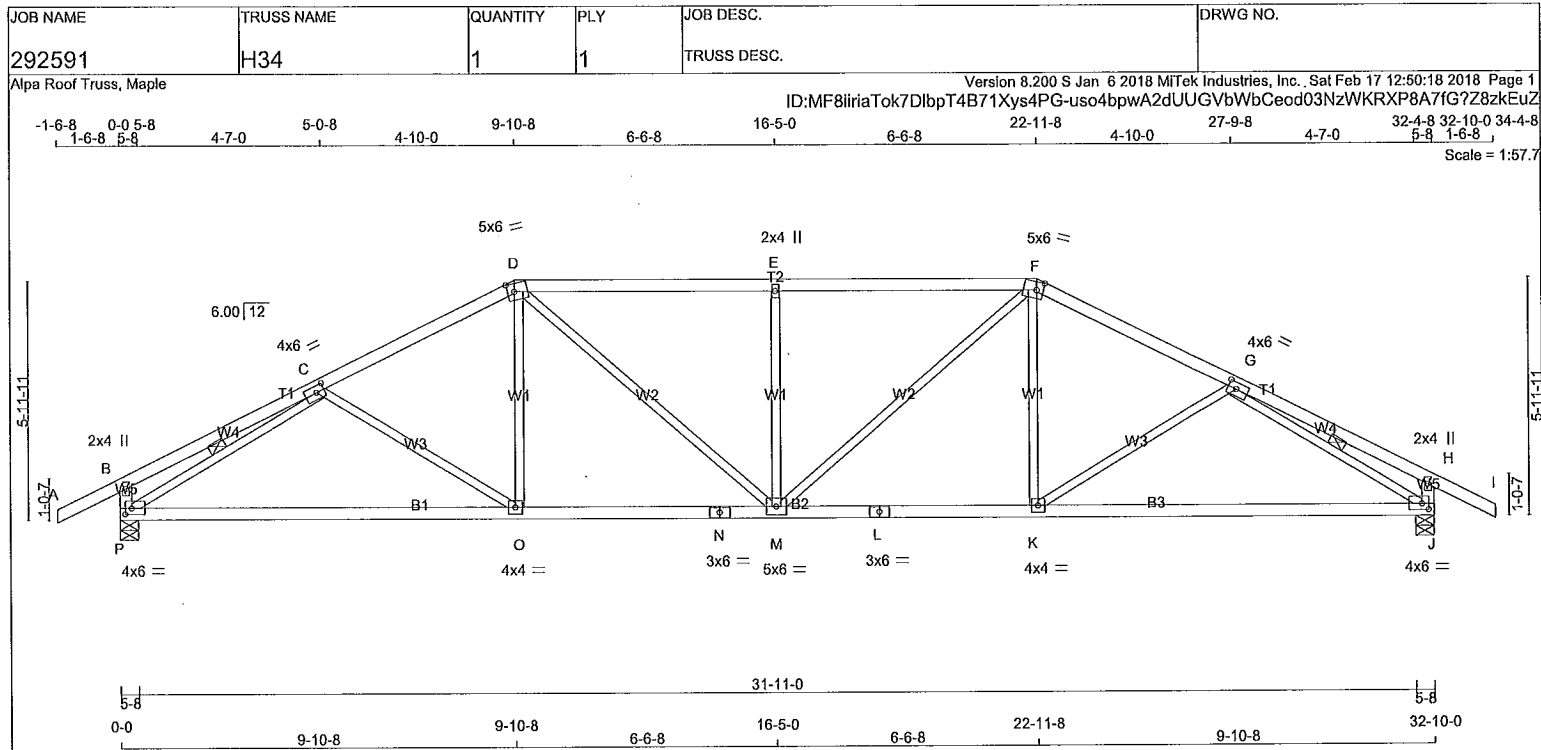
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A-18023471 CONTINUED ON PAGE 2





TOTAL WEIGHT = 128 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	DRY	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
P - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
P - N	2x4	DRY	No.2
N - L	2x4	DRY	No.2
L - J	2x4	DRY	No.2

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	MTWW-t	MT20	4.0	6.0	2.00	2.50
D	TTWW-m	MT20	5.0	6.0	2.50	2.00
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.50	2.00
G	MTWW-t	MT20	4.0	6.0	2.00	2.50
H	TMV+p	MT20	2.0	4.0		
J	BMVW1-t	MT20	4.0	6.0	1.75	2.00
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	5.0	6.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMVW1-t	MT20	4.0	6.0	1.75	2.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT		VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
P		1709	0	1709	0	0	5-8	1-14	
J		1709	0	1709	0	0	5-8	1-14	

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1216	757 / 0	0 / 0	0 / 0	0 / 0	459 / 0	0 / 0
J	1216	757 / 0	0 / 0	0 / 0	0 / 0	459 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.94 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-P, G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 MAX (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LC1 MAX (LC)	
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 28	-78.0	-78.0 0.14 (1)	10.00	C-O	-171 / 13	0.11 (1)
B-C	0 / 19	-78.0	-78.0 0.28 (1)	10.00	O-D	0 / 285	0.07 (4)
C-D	-2203 / 0	-78.0	-78.0 0.29 (1)	4.35	D-M	0 / 530	0.12 (1)
D-E	-2358 / 0	-78.0	-78.0 0.53 (1)	3.94	M-E	-626 / 0	0.35 (1)
E-F	-2358 / 0	-78.0	-78.0 0.53 (1)	3.94	M-F	0 / 530	0.12 (1)
F-G	-2203 / 0	-78.0	-78.0 0.29 (1)	4.35	K-F	0 / 285	0.07 (4)
G-H	0 / 19	-78.0	-78.0 0.28 (1)	10.00	K-G	-171 / 13	0.11 (1)
H-I	0 / 28	-78.0	-78.0 0.14 (1)	10.00	P-C	-2497 / 0	0.63 (1)
P-B	-277 / 0	0.0	0.0 0.03 (1)	7.81	G-J	-2497 / 0	0.63 (1)
J-H	-277 / 0	0.0	0.0 0.03 (1)	7.81			
P-O	0 / 2097	-18.5	-18.5 0.59 (4)	10.00			
O-N	0 / 1958	-18.5	-18.5 0.60 (4)	10.00			
N-M	0 / 1958	-18.5	-18.5 0.60 (4)	10.00			
M-L	0 / 1958	-18.5	-18.5 0.60 (4)	10.00			
L-K	0 / 1958	-18.5	-18.5 0.60 (4)	10.00			
K-J	0 / 2097	-18.5	-18.5 0.59 (4)	10.00			

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(56 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.09")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (1.09")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.36")

CSI: TC=0.53/1.00 (D-E:1), BC=0.60/1.00 (M-O:4), WB=0.63/1.00 (G-J:1), SSI=0.25/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

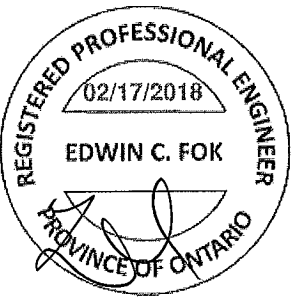
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

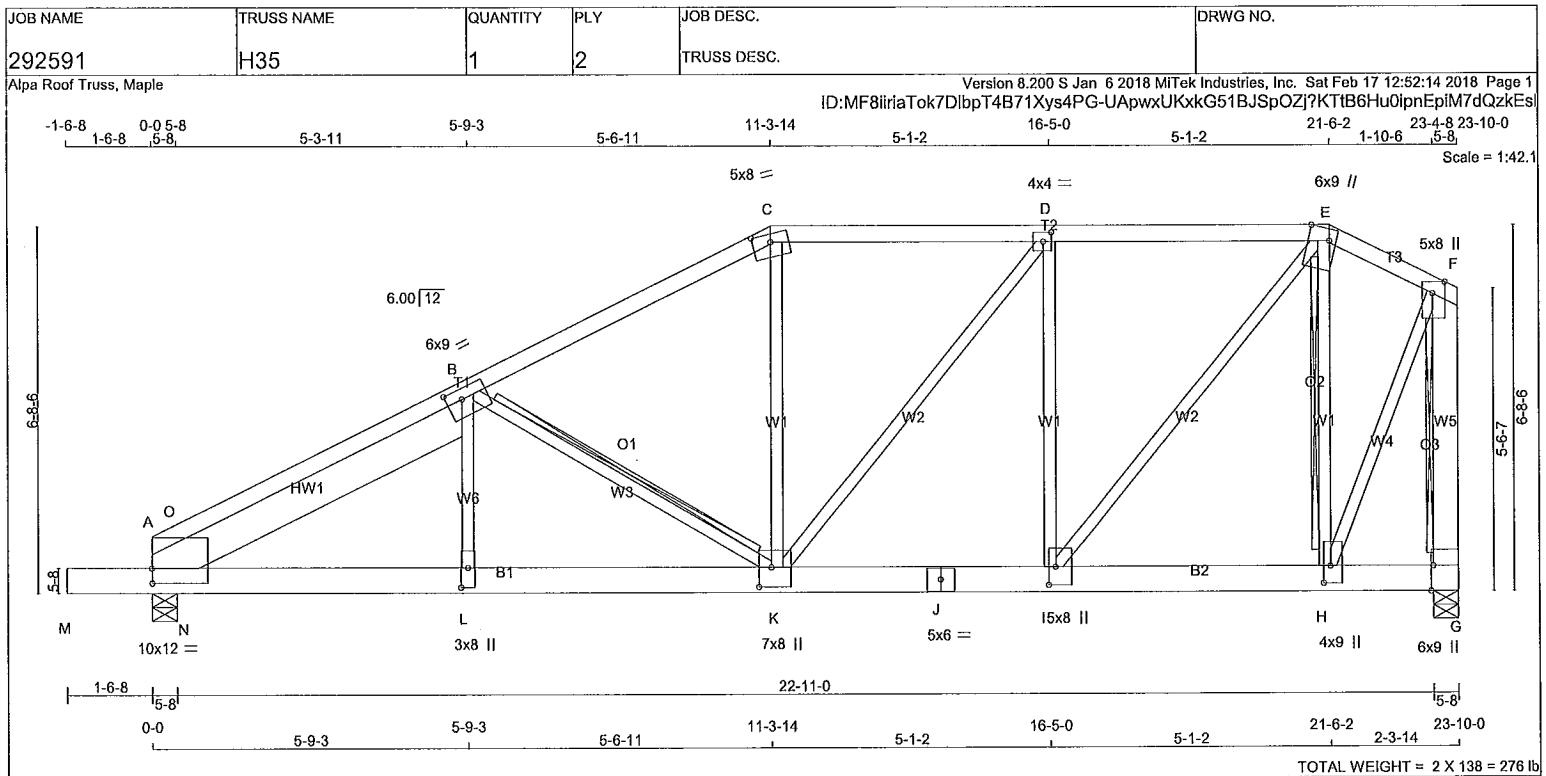
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (G) (INPUT = 0.90)
JSI METAL= 0.63 (N) (INPUT = 1.00)

A-18023473





LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4 DRY	No.2	SPF
C - E	2x4 DRY	No.2	SPF
E - F	2x4 DRY	No.2	SPF
G - F	2x6 DRY	No.2	SPF
M - J	2x6 DRY	1650F 1.5E	SPF
J - G	2x6 DRY	1650F 1.5E	SPF

REINFORCING MEMBERS			
HW1	2x8 DRY	No.2	SPF
ALL WEBS			
EXCEPT	2x3 DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	12	TOP
C - E	12	TOP
E - F	12	TOP
G - F	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
M - J	9	SIDE(282.4)
J - G	9	SIDE(282.4)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	
2x8	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
A	8030	0	8030	0	5-8
G	7881	0	7881	0	5-8

UNFACTORED REACTIONS		1ST LCASE	MAX/MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
A	5725	3494 / 0	0 / 0
G	5619	3429 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, G
BEARING SIZE FACTOR = 1.15 AT JNT(S) G (BASED ON SUPPORT DEPTH = 1-8)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.70 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT B-K, E-H, F-G

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 8" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED	FACTORED	W E B S	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX	MEMB.	FORCE (LBS)
FR-TO		FROM	TO	FR-TO	
A - O	-10379 / 0	-78.0	-78.0 0.42 (1)	L - B	0 / 3265
O - B	-6493 / 0	-78.0	-78.0 0.36 (1)	B - K	-3022 / 0
B - C	-8494 / 0	-78.0	-78.0 0.61 (1)	K - C	0 / 3444
C - D	-7637 / 0	-78.0	-78.0 0.42 (1)	K - D	0 / 1739
D - E	-6560 / 0	-78.0	-78.0 0.35 (1)	I - D	-1813 / 0
E - F	-3188 / 0	-78.0	-78.0 0.07 (1)	I - E	0 / 6005
F - G	-7561 / 0	0.0	0.0 0.63 (1)	H - E	-3502 / 0
				H - F	0 / 6674
				N - O	0 / 5476
				N - B	-5301 / 0
M - A	0 / 0	-96.5	-96.5 0.03 (1)		
A - N	0 / 5811	-583.4	-583.4 0.39 (1)		
N - L	0 / 10183	-583.4	-583.4 0.74 (1)		
L - K	0 / 10132	-583.4	-583.4 0.76 (1)		
K - J	0 / 6560	-583.4	-583.4 0.61 (1)		
J - I	0 / 6560	-583.4	-583.4 0.61 (1)		
I - H	0 / 2790	-583.4	-583.4 0.37 (1)		
H - G	0 / 0	-583.4	-583.4 0.24 (1)		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CstdGlrdr
START DISTANCE = 0-0
START SPAN CARRIED = 25-5-1
END DISTANCE = 23-10-0
END SPAN CARRIED = 25-5-1
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDTL LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.79")
CALCULATED VERT. DEFL.(LL) = L/999 (0.15")
ALLOWABLE DEFL.(TL) = L/360 (0.79")
CALCULATED VERT. DEFL.(TL) = L/972 (0.29")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL) = L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.63/1.00 (F-G:1), BC=0.76/1.00 (K-L:1), WB=0.83/1.00 (F-H:1), SSI=0.55/1.00 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 L.S BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

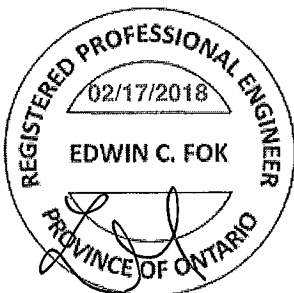
AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

A-18023474

CONTINUED ON PAGE 2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292591	H35	1	2	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Sat Feb 17 12:52:14 2018 Page 2
ID:MF8ilriaTok7DlbpT4B71Xys4PG-UApwxUKxkG51BJSpOZj?KTiB6Hu0lpnEpiM7dQzkEs

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBMW1-l	MT20	10.0	12.0	3.25	
B	TMWWWW-l	MT20	6.0	9.0	2.25	3.25
C	TTW-m	MT20	5.0	8.0	1.75	4.00
D	TMWW-l	MT20	4.0	4.0	2.00	1.75
E	TTWW+m	MT20	6.0	9.0	Edge	4.50
F	TMVW+p	MT20	5.0	8.0	Edge	2.75
G	BMV1+t	MT20	6.0	9.0	Edge	0.50
H	BMWW+l	MT20	4.0	9.0	3.75	1.50
I	BMWW+l	MT20	5.0	8.0	4.00	1.50
J	BS-t	MT20	5.0	6.0		
K	BMWWWW+t	MT20	7.0	8.0	4.25	2.75
L	BMW+w	MT20	3.0	8.0	4.25	1.50

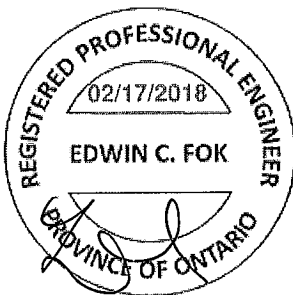
Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

PLATE PLACEMENT TOL. = 0.250 inches

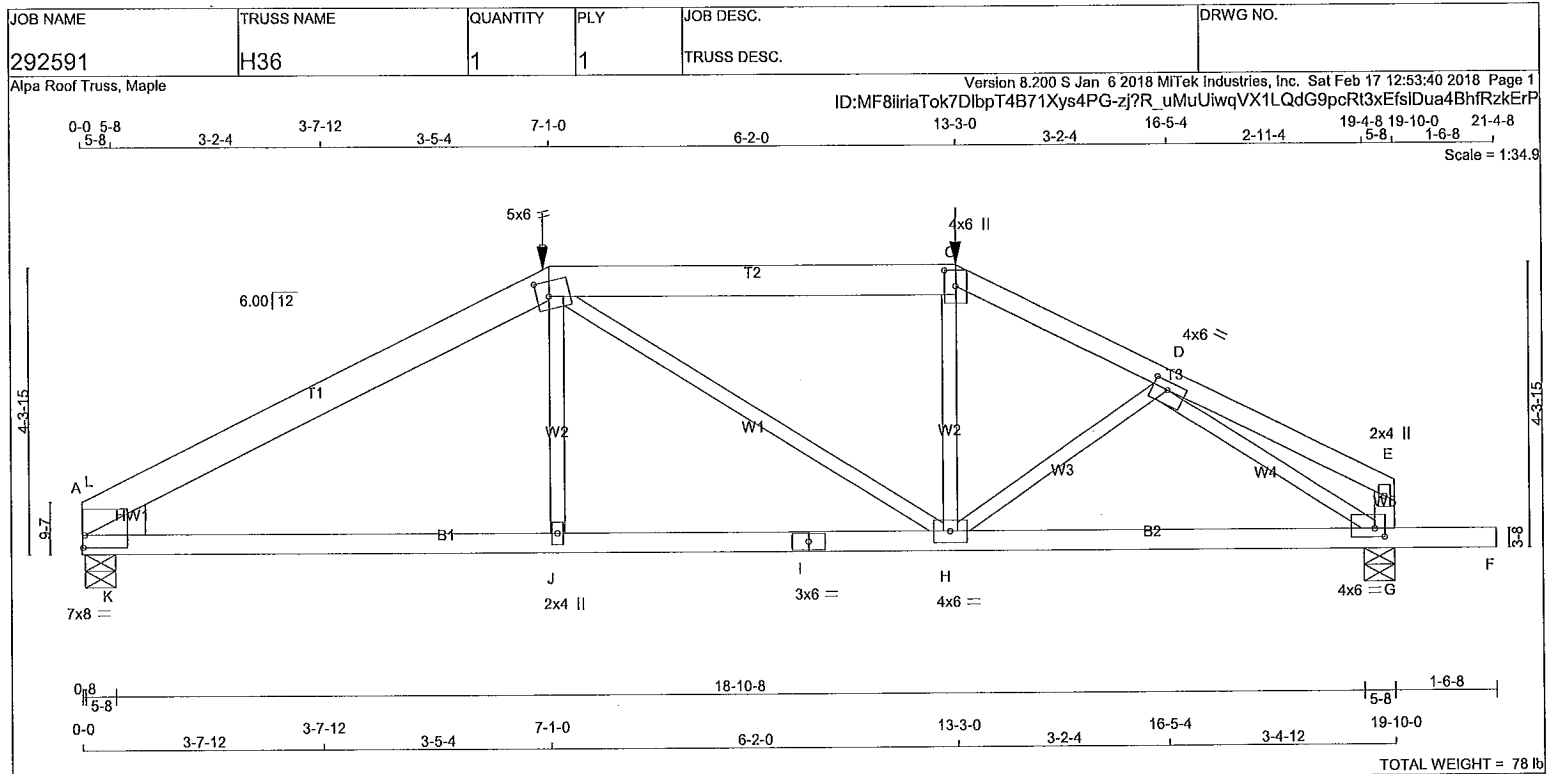
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (E) (INPUT = 0.90)
JSI METAL= 0.83 (J) (INPUT = 1.00)

NOTE: Lateral brace(s) shown shall be 1x4
for Part 9 design as per OBC 9.23.13.11,
and no less than 2x4 for Part 4 design



A.18023474(2)



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - B	2x6 DRY	No.2	SPF
B - C	2x6 DRY	No.2	SPF
C - E	2x4 DRY	No.2	SPF
G - E	2x4 DRY	No.2	SPF
A - I	2x4 DRY	No.2	SPF
I - F	2x4 DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMBH1-I	MT20	7.0	8.0	2.25 0.25
B	TTWW-m	MT20	5.0	6.0	2.75 2.25
C	TTW+p	MT20	4.0	6.0	3.00 2.00
D	TMWW-t	MT20	4.0	6.0	1.50 2.75
E	TMV+p	MT20	2.0	4.0	
G	BMVW1-t	MT20	4.0	6.0	1.50 1.75
H	BMVWW-t	MT20	4.0	6.0	
I	BS-t	MT20	3.0	6.0	
J	BMV+w	MT20	2.0	4.0	

HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 434.9 lbs FACTORED DOWN AT 13-3-0, AND 467.9 lbs FACTORED DOWN AT 7-1-0 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
	VERT	HORZ	DOWN	HORZ			
A	1881	0	1881	0	0	2-1	2x6 L
G	2056	0	2056	0	0	5-8	2-3

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM.LIVE				
A	1344	801/0	0/0	0/0	0/0	544/0	0/0
G	1469	879/0	0/0	0/0	0/0	589/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MAX. FACTORED		FACTORED			MAX. UNBRAC	MAX. FACTORED		MAX. CSI (LC)	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	LC1 MAX		MEMB.	FORCE (LBS)		
FR-TO		FROM	TO	CSI (LC)	LENGTH	FR-TO			
A-L	-3443 / 0	-78.0	-78.0	0.47 (1)	4.00	J-B	0 / 264	0.10 (4)	
L-B	-3002 / 0	-78.0	-78.0	0.51 (1)	4.27	B-H	-119 / 0	0.12 (1)	
B-C	-2555 / 0	-167.3	-167.3	0.65 (1)	4.23	H-C	0 / 257	0.10 (4)	
C-D	-2847 / 0	-78.0	-78.0	0.22 (1)	3.91	H-D	0 / 238	0.06 (1)	
D-E	0 / 8	-78.0	-78.0	0.13 (1)	10.00	D-G	-2897 / 0	0.88 (1)	
G-E	-115 / 0	0.0	0.0	0.01 (1)	7.81	K-L	0 / 625	0.00 (1)	
A-K	0 / 2649	-39.7	-39.7	0.79 (1)	10.00				
K-J	0 / 2649	-39.7	-39.7	0.79 (1)	10.00				
J-I	0 / 2655	-39.7	-39.7	0.71 (1)	10.00				
I-H	0 / 2655	-39.7	-39.7	0.71 (1)	10.00				
H-G	0 / 2359	-39.7	-39.7	0.67 (4)	10.00				
G-F	0 / 0	-96.5	-96.5	0.17 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
B	7-1-0	-468	-468	---	FRONT	VERT	TOTAL
C	13-3-0	-435	-435	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 21.0 PSF
	DL	= 6.0 PSF
BOT CH.	LL	= 0.0 PSF
	DL	= 7.4 PSF
TOTAL LOAD		= 34.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 7-1-0
RIGHT SETBACK = 6-7-0
END SETBACK = 6-7-0
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDTL LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL)= L/999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL)= L/999 (0.18")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL)= L/999 (0.01")

CSI: TC=0.85/1.00 (B-C:1), BC=0.79/1.00 (A-K:1), WB=0.88/1.00 (D-G:1), SSI=0.51/1.00 (A-K:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

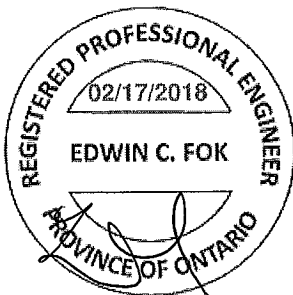
COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

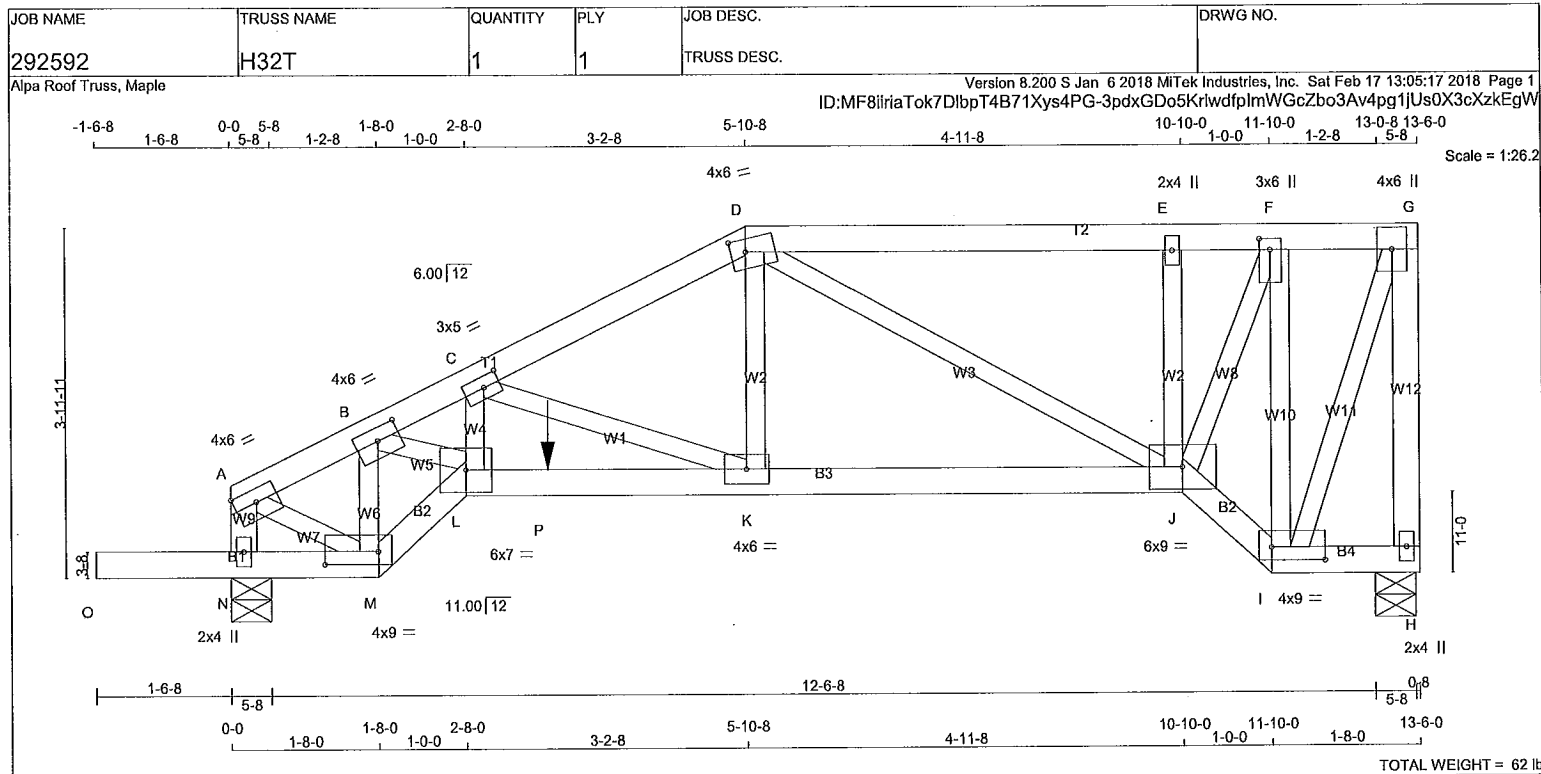
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

A-18023475

CONTINUED ON PAGE 2



A-18023476



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
H - I	2x4	DRY	No.2
N - A	2x4	DRY	No.2
O - M	2x4	DRY	No.2
M - L	2x4	DRY	No.2
L - J	2x4	DRY	No.2
J - I	2x4	DRY	No.2
I - H	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0	1.75	Edge
B	TMVW-t	MT20	4.0	6.0	1.75	3.00
C	TMVW-t	MT20	3.0	5.0	1.50	2.25
D	TTVW-m	MT20	4.0	6.0	1.75	2.00
E	TMVW-w	MT20	2.0	4.0		
F	TMVW-t	MT20	3.0	6.0	1.50	1.50
G	TMVW+p	MT20	4.0	6.0		
H	BMV1+p	MT20	2.0	4.0		
I	BBWW-I	MT20	4.0	9.0	1.75	7.25
J	BBWWW-p	MT20	6.0	9.0		
K	BMWW-t	MT20	4.0	6.0		
L	BBWW-I	MT20	6.0	7.0		
M	BBWW-I	MT20	4.0	9.0	1.75	7.25
N	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1)

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
H	1344	0	1344	0
N	1448	0	1448	0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	956	595 / 0	0 / 0	0 / 0	0 / 0	0 / 0	361 / 0	0 / 0
N	1027	657 / 0	0 / 0	0 / 0	0 / 0	0 / 0	369 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.87 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)	UNBRAC LENGTH
FR-TO		FROM	TO	FR-TO			
A-B	-1380 / 0	-78.0	-78.0 0.10 (1)	5.41	M-B	-1583 / 0	0.25 (1)
B-C	-2980 / 0	-78.0	-78.0 0.17 (1)	3.87	B-L	0 / 1760	0.44 (1)
C-D	-1995 / 0	-78.0	-78.0 0.20 (1)	4.56	L-C	0 / 581	0.14 (1)
D-E	-1131 / 0	-78.0	-78.0 0.28 (1)	5.59	C-K	-903 / 0	0.21 (1)
E-F	-1112 / 0	-153.5	-153.5 0.26 (1)	5.63	K-D	0 / 998	0.25 (1)
F-G	-529 / 0	-153.5	-153.5 0.12 (1)	6.25	D-J	-787 / 0	0.41 (1)
H-G	-1313 / 0	0.0	0.0 0.33 (1)	7.02	J-E	-493 / 0	0.10 (1)
N-A	-1215 / 0	0.0	0.0 0.13 (1)	7.23	J-F	0 / 1677	0.42 (1)
					I-F	-1558 / 0	0.40 (1)
O-N	0 / 0	-96.5	-96.5 0.17 (1)	10.00	I-G	0 / 1276	0.32 (1)
N-M	-1 / 0	-18.5	-18.5 0.17 (1)	10.00	A-M	0 / 1327	0.33 (1)
M-L	0 / 1509	-18.5	-18.5 0.27 (1)	10.00			
L-P	0 / 2829	-18.5	-18.5 0.91 (1)	10.00			
P-K	0 / 2829	-112.0	-112.0 0.91 (1)	10.00			
K-J	0 / 1820	-112.0	-112.0 0.77 (1)	10.00			
J-I	0 / 658	-36.4	-36.4 0.12 (1)	10.00			
I-H	0 / 0	-36.4	-36.4 0.03 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
P	3-7-4	-407	-407	-	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 21.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 5-10-8
RIGHT SETBACK = 0-0
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDTL. LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 2-8-0 OF SPAN MEASURED FROM THE RIGHT.

GIRDER TYPE: CStdGirder
START DISTANCE = 3-7-4
START SPAN CARRIED = 5-10-8
END DISTANCE = 10-10-0
END SPAN CARRIED = 5-10-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDTL. LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***
ADDTL. USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

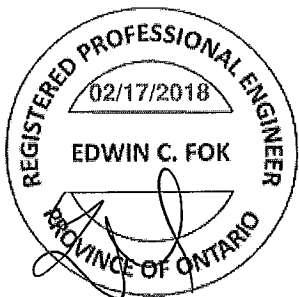
(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.45")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (0.45")
CALCULATED VERT. DEFL.(TL) = L/997 (0.16")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL) = L/120 (0.19")

A-18023477

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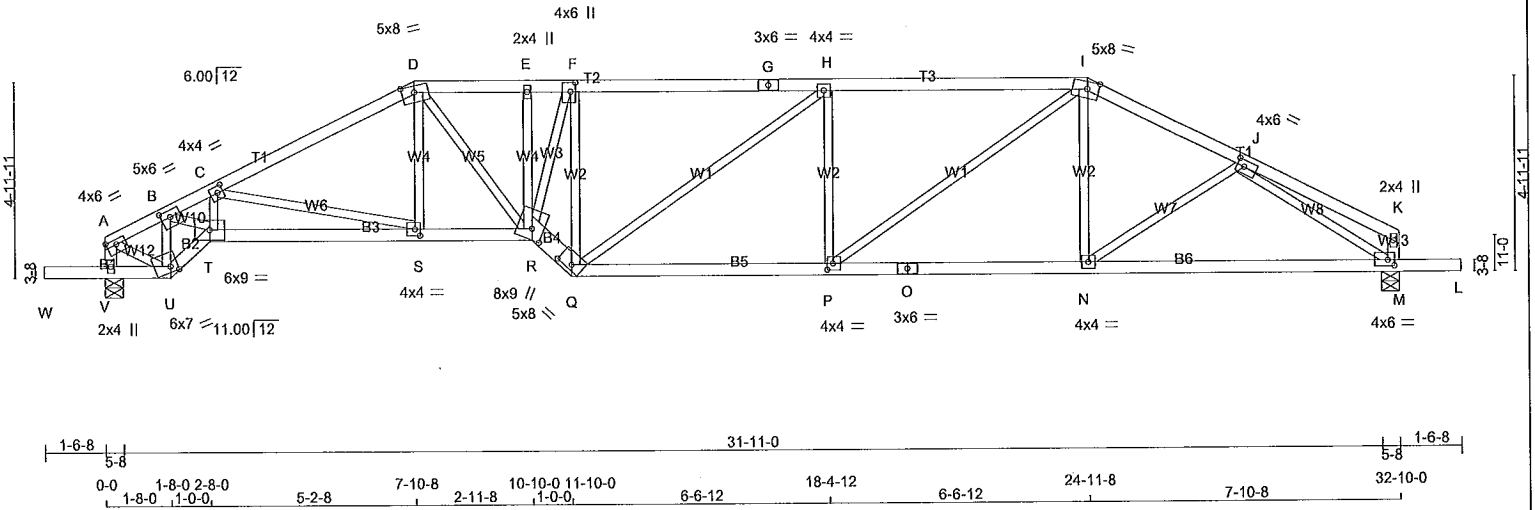


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292592	H33T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 8.200 S Jan 6 2018 MITek Industries, Inc. Sat Feb 17 13:06:14 2018 Page 1
ID:MF8ilriaTok7DlbpT4B71Xys4PG-unZUEIUrVmrzPgGb_o_NcfMucmWhEtzOra6kzkEfd

1-6-8 0-0 5-8 1-8-0 2-8-0 5-2-8 7-10-8 2-11-8 10-10-0 11-10-0 6-6-12 18-4-12 6-6-12 24-11-8 3-10-0 28-9-8 3-7-0 32-4-8 32-10-0 34-4-8 5-8 1-6-8
Scale = 1:58.5



TOTAL WEIGHT = 135 lb [M][F]

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - K	2x4	DRY	No.2
V - A	2x4	DRY	No.2
M - K	2x4	DRY	No.2
W - U	2x4	DRY	No.2
U - T	2x4	DRY	No.2
T - R	2x4	DRY	No.2
R - Q	2x6	DRY	No.2
Q - O	2x4	DRY	No.2
O - L	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
A	TMVW-t	MT20	4.0	6.0 1.50 3.00
B	TMVW-t	MT20	5.0	6.0 2.00 2.75
C	TMVW-t	MT20	4.0	4.0 2.00 1.75
D	TTWW-m	MT20	5.0	8.0 2.00 3.75
E	TMVW-w	MT20	2.0	4.0
F	TMVW-t	MT20	4.0	6.0 2.75 1.50
G	TS-t	MT20	3.0	6.0
H	TMVW-t	MT20	4.0	4.0
I	TTWW-m	MT20	5.0	8.0 2.25 3.50
J	TMVW-t	MT20	4.0	6.0 2.00 2.25
K	TMV+p	MT20	2.0	4.0
M	BMVW-t	MT20	4.0	6.0 1.75 2.00
N	BMVW-t	MT20	4.0	4.0
O	BS-t	MT20	3.0	6.0
P	BMVW-t	MT20	4.0	4.0 2.00 1.75
Q	BBVW-h	MT20	5.0	8.0 1.50 4.50
R	BBVW+m	MT20	8.0	9.0 3.25 3.50
S	BMVW-t	MT20	4.0	4.0 2.00 1.50
T	BBVW-t	MT20	6.0	9.0
U	BBVW-m	MT20	6.0	7.0 1.75 2.25
V	BMV1+p	MT20	2.0	4.0

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	2-9	1-13
V	1736	0	1736	0	0	5-8	5-8		
M	1729	0	1729	0	0	5-8	1-13		

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
V	1238	755 / 0	0 / 0	0 / 0	0 / 0	482 / 0	0 / 0		
M	1233	752 / 0	0 / 0	0 / 0	0 / 0	480 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.41 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	-1712 / 0	-78.0 -78.0 0.13 (1)	4.97	U-B	-1945 / 0	0.28 (1)	
B-C	-3802 / 0	-78.0 -78.0 0.28 (1)	3.41	B-T	0 / 2353	0.53 (1)	
C-D	-2813 / 0	-78.0 -78.0 0.39 (1)	3.83	T-C	0 / 255	0.06 (1)	
D-E	-3092 / 0	-78.0 -78.0 0.23 (1)	3.80	C-S	-966 / 0	0.47 (1)	
E-F	-3093 / 0	-78.0 -78.0 0.30 (1)	3.72	S-D	0 / 225	0.05 (4)	
F-G	-2637 / 0	-78.0 -78.0 0.51 (1)	3.81	D-R	0 / 927	0.21 (1)	
G-H	-2637 / 0	-78.0 -78.0 0.51 (1)	3.81	R-E	0 / 59	0.01 (1)	
H-I	-2812 / 0	-78.0 -78.0 0.55 (1)	3.65	R-F	0 / 1759	0.40 (1)	
I-J	-2298 / 0	-78.0 -78.0 0.19 (1)	4.37	Q-F	-2154 / 0	0.78 (1)	
J-K	0 / 13	-78.0 -78.0 0.16 (1)	10.00	Q-H	-214 / 0	0.28 (1)	
V-A	-1503 / 0	0.0 0.0 0.15 (1)	6.70	P-H	-459 / 0	0.17 (1)	
M-K	-125 / 0	0.0 0.0 0.01 (1)	7.81	P-I	0 / 941	0.21 (1)	
W-V	0 / 0	-96.5 -96.5 0.16 (1)	10.00	N-I	0 / 150	0.05 (4)	
V-U	-1 / 0	-18.5 -18.5 0.12 (1)	10.00	A-U	0 / 1639	0.37 (1)	
U-T	0 / 1864	-18.5 -18.5 0.30 (1)	10.00	J-M	-2469 / 0	0.92 (1)	
T-S	0 / 3446	-18.5 -18.5 0.62 (1)	10.00				
S-R	0 / 2511	-18.5 -18.5 0.46 (1)	10.00				
R-Q	0 / 3511	-18.5 -18.5 0.42 (1)	10.00				
Q-P	0 / 2812	-18.5 -18.5 0.55 (1)	10.00				
P-O	0 / 2045	-18.5 -18.5 0.46 (1)	10.00				
O-N	0 / 2045	-18.5 -18.5 0.46 (1)	10.00				
N-M	0 / 2037	-18.5 -18.5 0.47 (1)	10.00				
M-L	0 / 0	-96.5 -96.5 0.16 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.09")
CALCULATED VERT. DEFL.(LL) = L/999 (0.19")
ALLOWABLE DEFL.(TL)= L/360 (1.09")
CALCULATED VERT. DEFL.(TL) = L/959 (0.41")

CANTILEVER DEFLECTION:

ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/427 (0.04")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/216 (0.09")

CSI: TC=0.55/1.00 (H-I:1), BC=0.62/1.00 (S-T:1), WB=0.92/1.00 (J-M:1), SSI=0.24/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

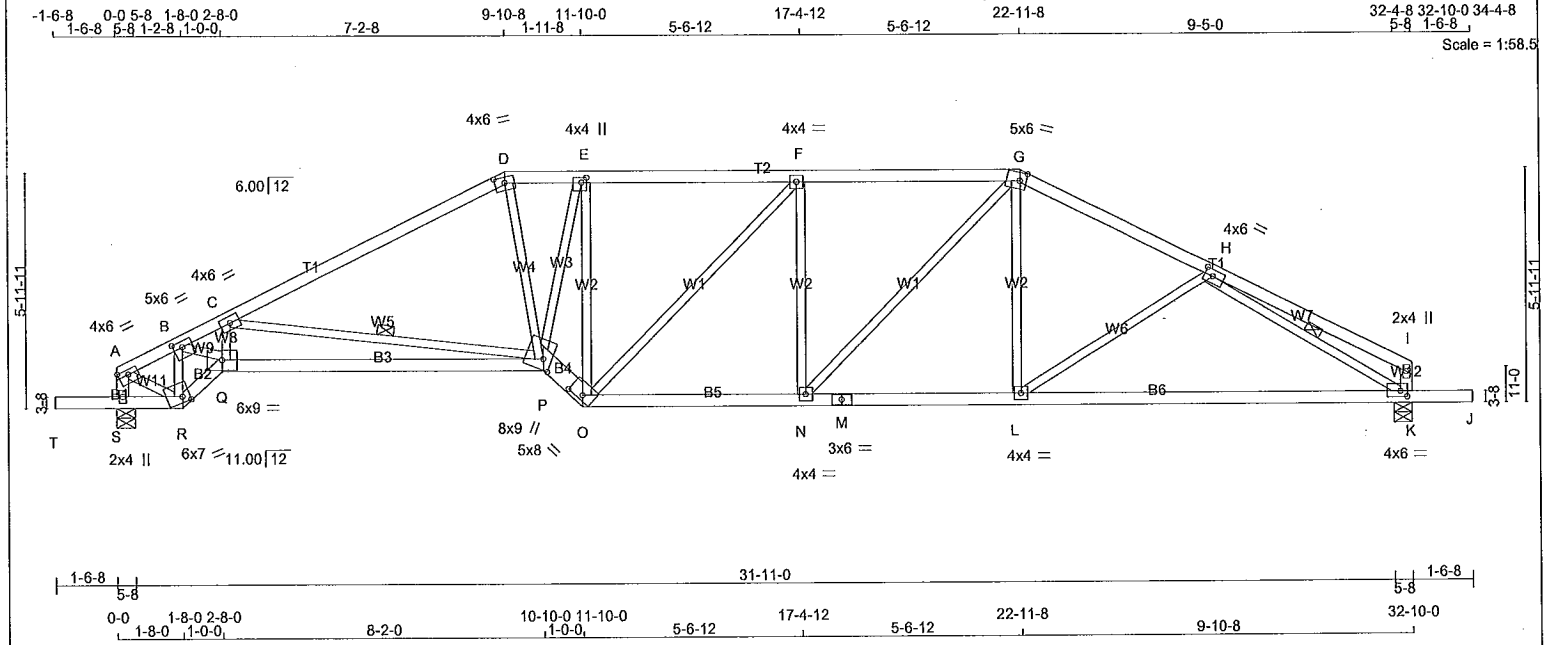
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (Q) (INPUT = 0.90)
JSI METAL= 0.61 (J) (INPUT = 1.00)

A-18023478

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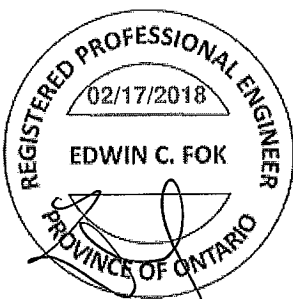


LUMBER				
N. L. G. A.		RULES		
CHORDS	SIZE	LUMBER		DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
S - A	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
R - Q	2x4	DRY	No.2	SPF
Q - P	2x4	DRY	No.2	SPF
P - O	2x6	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW-t	MT20	4.0	6.0	1.50 3.00
B	TMVW-t	MT20	5.0	6.0	1.75 2.75
C	TMVW-t	MT20	4.0	6.0	
D	TTW-m	MT20	4.0	6.0	1.75 3.00
E	TMVW+t	MT20	4.0	4.0	1.50 1.50
F	TMVW-t	MT20	4.0	4.0	
G	TTW-m	MT20	5.0	6.0	2.50 2.00
H	TMVW-t	MT20	4.0	6.0	2.00 2.50
I	TMV+p	MT20	2.0	4.0	
K	BMVW1-t	MT20	4.0	6.0	1.75 2.00
L	BMVW-t	MT20	4.0	4.0	
M	BS-t	MT20	3.0	6.0	
N	BMVW-t	MT20	4.0	4.0	
O	BBVW-h	MT20	5.0	8.0	1.50 4.50
P	BMVWVW+m	MT20	8.0	9.0	3.25 2.50
Q	BBVW-t	MT20	6.0	9.0	
R	BMVW-m	MT20	6.0	7.0	1.75 2.25
S	BMV1+p	MT20	2.0	4.0	

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			MAXIMUM FACTORED			INPUT	REQD
GROSS REACTION			GROSS REACTION			BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
S	1736	0	1736	0	0	5-8	2-9
K	1729	0	1729	0	0	5-8	1-13

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1238	755 / 0	0 / 0	0 / 0	0 / 0	482 / 0	0 / 0
K	1233	752 / 0	0 / 0	0 / 0	0 / 0	480 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.10 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-P, H-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	-1692 / 0	-78.0	-78.0 0.16 (1)	4.97	R-B	-1910 / 0	0.28 (1)
B-C	-3891 / 0	-78.0	-78.0 0.49 (1)	3.10	B-Q	0 / 2559	0.58 (1)
C-D	-2554 / 0	-78.0	-78.0 0.69 (1)	3.67	Q-C	0 / 153	0.05 (4)
D-E	-2443 / 0	-78.0	-78.0 0.17 (1)	4.27	C-P	-1362 / 0	0.61 (1)
E-F	-2150 / 0	-78.0	-78.0 0.34 (1)	4.32	D-P	0 / 884	0.20 (1)
F-G	-2319 / 0	-78.0	-78.0 0.36 (1)	4.17	P-E	0 / 1411	0.32 (1)
G-H	-2195 / 0	-78.0	-78.0 0.29 (1)	4.35	Q-E	-1681 / 0	0.93 (1)
H-I	0 / 18	-78.0	-78.0 0.28 (1)	10.00	O-F	-240 / 0	0.30 (1)
S-A	-1503 / 0	0.0	0.0 0.15 (1)	6.70	N-F	-330 / 0	0.18 (1)
K-I	-156 / 0	0.0	0.0 0.02 (1)	7.81	N-G	0 / 528	0.12 (1)
					L-G	0 / 281	0.07 (4)
T-S	0 / 0	-96.5	-96.5 0.16 (1)	10.00	L-H	-177 / 10	0.11 (1)
S-R	-1 / 0	-18.5	-18.5 0.12 (1)	10.00	A-R	0 / 1609	0.36 (1)
R-Q	0 / 1830	-18.5	-18.5 0.30 (1)	10.00	H-K	-2489 / 0	0.64 (1)
Q-P	0 / 3615	-18.5	-18.5 0.79 (1)	10.00			
P-O	0 / 2875	-18.5	-18.5 0.34 (1)	10.00			
Q-N	0 / 2320	-18.5	-18.5 0.46 (1)	10.00			
N-M	0 / 1950	-18.5	-18.5 0.58 (4)	10.00			
M-L	0 / 1950	-18.5	-18.5 0.58 (4)	10.00			
L-K	0 / 2092	-18.5	-18.5 0.57 (4)	10.00			
K-J	0 / 0	-96.5	-96.5 0.16 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.09")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (1.09")
CALCULATED VERT. DEFL.(TL) = L/ 809 (0.49")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= $1/120$ (0.19")
CALCULATED VERT. DEFL.(LL) = $1/344$ (0.05")
ALLOWABLE DEFL.(TL)= $1/120$ (0.19")
CALCULATED VERT. DEFL.(TL) = $1/177$ (0.10")

CSI: TC=0.69/1.00 (C-D:1), BC=0.79/1.00 (P-Q:1)
WB=0.93/1.00 (E-O:1), SSI=0.29/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)	(PLI)	(PSI)	(PLI)	(PSI)	(PLI)
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.63 (Q) (INPUT = 1.00)

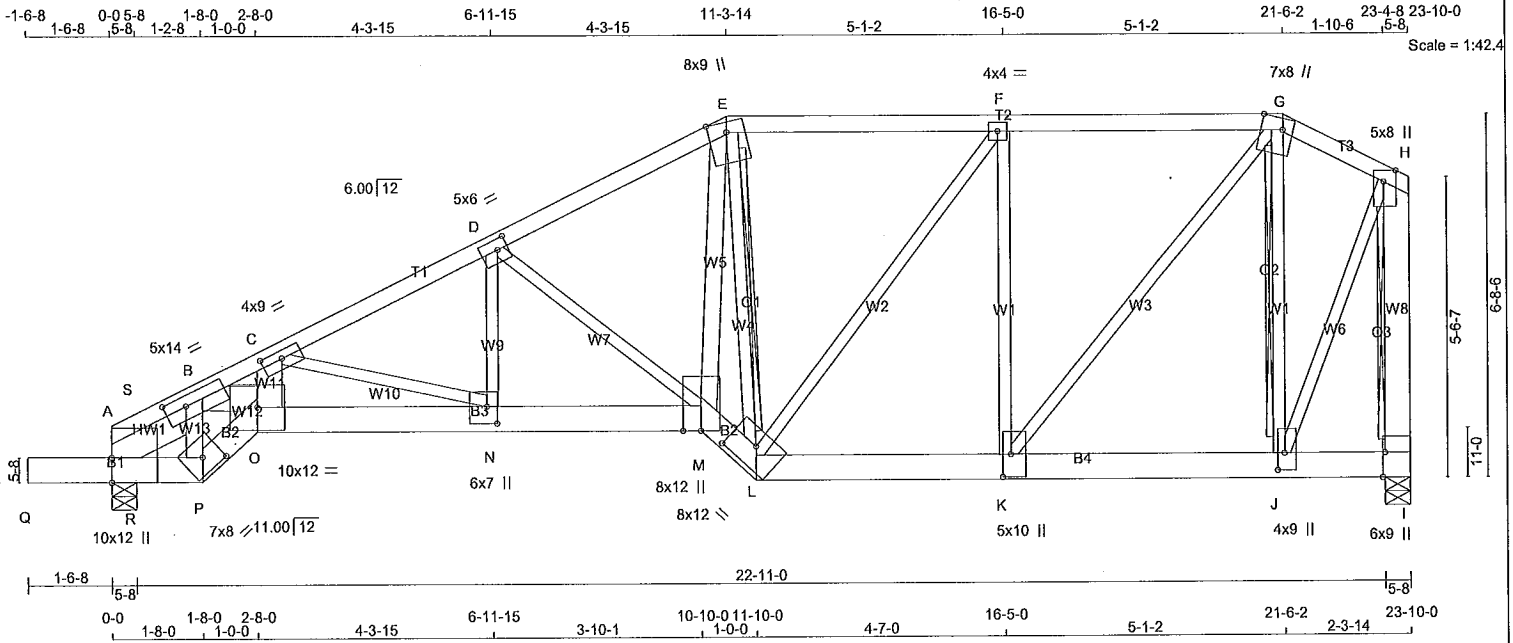
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292592	H35T	1	2	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Sat Feb 17 13:07:51 2018 Page 1
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TOTAL WEIGHT = 2 X 140 = 281 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - E	2x4	DRY	No.2	2100F 1.8E	SPF
E - G	2x4	DRY	No.2	1650F 1.5E	SPF
G - H	2x4	DRY	No.2	1650F 1.5E	SPF
I - H	2x6	DRY	No.2	No.2	SPF
Q - P	2x6	DRY	No.2	No.2	SPF
P - O	2x6	DRY	No.2	No.2	SPF
O - M	2x6	DRY	No.2	1650F 1.5E	SPF
M - L	2x6	DRY	No.2	No.2	SPF
L - I	2x6	DRY	No.2	No.2	SPF

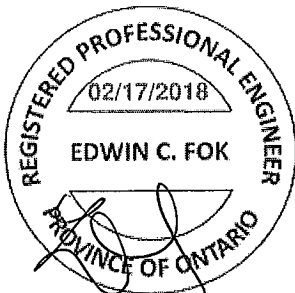
REINFORCING MEMBERS	HW1	SIZE	LUMBER	DESCR.
HW1	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
B - O	2x6	DRY	No.2	SPF
O - C	2x6	DRY	No.2	SPF
M - E	2x4	DRY	1650F 1.5E	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - E	12	TOP
E - G	12	TOP
G - H	12	TOP
I - H	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
Q - P	9	SIDE(282.4)
P - O	12	SIDE(208.2)
O - M	9	SIDE(282.4)
M - L	12	SIDE(208.2)
L - I	9	SIDE(282.4)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	
2x4	6	
2x6	6	

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
A	8035	0	0	5-8
I	7876	0	0	5-8

ALLOW FOR 0.3" OF HORIZONTAL MOVEMENT DUE TO TOTAL LOAD

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS
JT	COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL
A	5728 3496 / 0 0 / 0 0 / 0 2232 / 0 0 / 0
I	5615 3427 / 0 0 / 0 0 / 0 2188 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, I
BEARING SIZE FACTOR = 1.15 AT JNT(S) A, I (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.08 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x6 DRY SPF No.2 T-BRACE AT E-L
2x4 DRY SPF No.2 T-BRACE AT G-J, H-I

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC2)	MAX. FACTORED VERT. LOAD (LC3)	MAX. FACTORED VERT. LOAD (LC4)
FR-TO						
A-S	-6162 / 0	-78.0	-78.0	0.41 (1)	4.28	P-B -6805 / 0 0.38 (1)
S-B	-4749 / 0	-78.0	-78.0	0.31 (1)	4.94	B-O 0 / 10837 0.70 (1)
B-C	-19858 / 0	-78.0	-78.0	0.68 (1)	2.08	O-C 0 / 3849 0.25 (1)
C-D	-13757 / 0	-78.0	-78.0	0.43 (1)	3.09	C-N -4943 / 0 0.76 (1)
D-E	-10182 / 0	-78.0	-78.0	0.22 (1)	3.77	N-D 0 / 3854 0.48 (1)
E-F	-7464 / 0	-78.0	-78.0	0.26 (1)	3.96	D-M -4232 / 0 0.91 (1)
F-G	-6593 / 0	-78.0	-78.0	0.24 (1)	4.19	M-E 0 / 11454 0.73 (1)
G-H	-3176 / 0	-78.0	-78.0	0.05 (1)	5.79	E-L -7174 / 0 0.95 (1)
H-I	-7531 / 0	0.0	0.0	0.63 (1)	7.81	L-F 0 / 1476 0.18 (1)
						K-F -1654 / 0 0.59 (1)
Q-A	0 / 0	-96.5	-96.5	0.04 (1)	10.00	K-G 0 / 6078 0.75 (1)
A-R	0 / 3911	-583.4	-583.4	0.58 (1)	10.00	J-G -3558 / 0 0.68 (1)
R-P	0 / 8244	-583.4	-583.4	0.63 (1)	10.00	J-H 0 / 6647 0.82 (1)
P-O	0 / 11675	-583.4	-583.4	0.78 (1)	10.00	R-S 0 / 2111 0.00 (1)
O-N	0 / 17095	-583.4	-583.4	0.95 (1)	10.00	R-B -6486 / 0 0.26 (1)
N-M	0 / 12343	-583.4	-583.4	0.71 (1)	10.00	
M-L	0 / 10914	-583.4	-583.4	0.74 (1)	10.00	
L-K	0 / 6591	-583.4	-583.4	0.88 (1)	10.00	
K-J	0 / 2778	-583.4	-583.4	0.64 (1)	10.00	
J-I	0 / 0	-583.4	-583.4	0.32 (1)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 25-5-1
END DISTANCE = 23-10-0
END SPAN CARRIED = 25-5-1
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADD'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.I. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.79")
CALCULATED VERT. DEFL.(LL) = L/999 (0.24")
ALLOWABLE DEFL.(TL) = L/360 (0.79")
CALCULATED VERT. DEFL.(TL) = L/596 (0.48")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL) = L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/203 (0.09")
ALLOWABLE DEFL.(TL) = L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/120 (0.18")

CSI: TC=0.68/1.00 (B-C:1), BC=0.95/1.00 (N-O:1)
WB=0.95/1.00 (E-L:1), SSI=0.96/1.00 (A-R:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

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CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292592	H35T	1	2	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 8.200 S Jan 6 2018 MITek Industries, Inc. Sat Feb 17 13:07:51 2018 Page 2
ID:MF8iiriaTok7DlbpT4B71Xys4PG-ZRw9btgh3A845Jbvnrljp3Blit_eHIZm9G_rD1czkEe6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBMW1+t	MT20	10.0	12.0	Edge	
B	TMWWW-t	MT20	5.0	14.0	2.25	4.75
C	TMWW-t	MT20	4.0	9.0	1.75	4.50
D	TMWW-t	MT20	5.0	6.0	2.25	2.25
E	TTWW+m	MT20	8.0	9.0	Edge	
F	TMWW-t	MT20	4.0	4.0		
G	TTWW+m	MT20	7.0	8.0	Edge	4.75
H	TMVV+p	MT20	5.0	8.0	Edge	2.75
I	BMV1+t	MT20	6.0	9.0	Edge	0.50
J	BMWW+t	MT20	4.0	9.0	3.75	1.50
K	BMWW+t	MT20	5.0	10.0	5.00	1.75
L	BBWW-h	MT20	8.0	12.0	4.50	6.00
M	BBWW+p	MT20	8.0	12.0		
N	BMWW+t	MT20	6.0	7.0	3.75	2.25
O	BBWW-l	MT20	10.0	12.0		
P	BBW-h	MT20	7.0	8.0	3.25	4.00

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

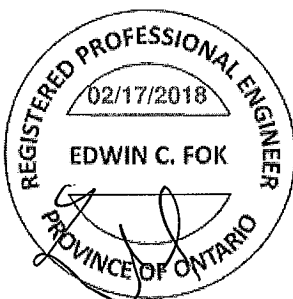
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (D) (INPUT = 0.90)
JSI METAL= 0.88 (E) (INPUT = 1.00)

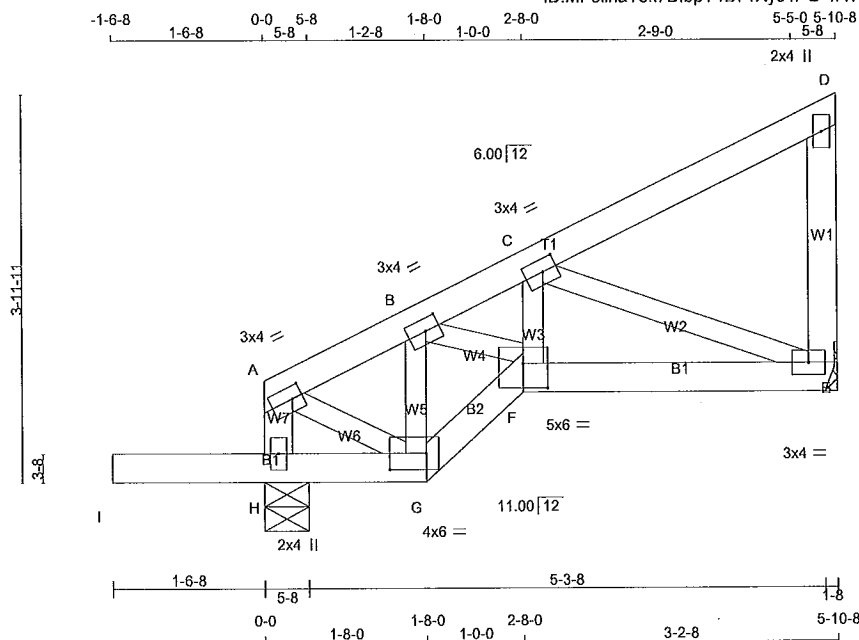
NOTE: Lateral brace(s) shown shall be 1x4
for Part 9 design as per OBC 9.23.13.11,
and no less than 2x4 for Part 4 design



A-18023480(2)

Alpa Roof Truss, Maple

Version 8.200 S Jan 6 2018 MITek Industries, Inc. Sat Feb 17 13:08:26 2018 Page 1
ID:MF8IirjaTok7DIbpT4B71Xys4PG-1AYNxr5eqwZ6oimdKIAxjS7QG5nrSb7JJwpaI4zkEdZ



Scale = 1:23.6

TOTAL WEIGHT = 3 X 26 = 79 lb

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-1	MT20	3.0	4.0	1.50	1.25
B	TMVW-1	MT20	3.0	4.0	1.50	1.75
C	TMVW-1	MT20	3.0	4.0	1.50	1.75
D	TMV-p	MT20	2.0	4.0		
E	BMVW-1-t	MT20	3.0	4.0		
F	BBWW-1	MT20	5.0	6.0	3.00	3.00
G	BBWW-1	MT20	4.0	6.0	2.00	4.50
H	BMV-1+p	MT20	2.0	4.0		

BEARINGS		FACTORED	MAXIMUM FACTORED		INPUT	REQRD
JT	VERT	HORZ	DOWN	HORZ	BRG	BRG
H	452	0	452	0	0	1-8
E	264	0	264	0	0	1-8

1ST LCASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	322	197 / 0	0 / 0	0 / 0	0 / 0	125 / 0	0 / 0
E	188	115 / 0	0 / 0	0 / 0	0 / 0	73 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

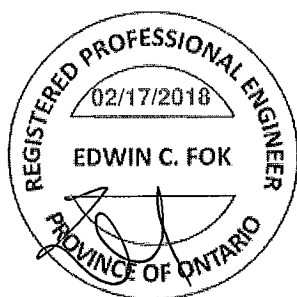
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY
APPLIED

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MAX. FACTORED		FACTORED		MAX. FACTORED				
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM TO			FR-TO			
H-A	-219 / 0	0.0	0.0	0.02 (1)	7.81	A-G	0 / 184	0.04 (1)
A-B	-191 / 0	-78.0	-78.0	0.03 (1)	6.25	G-B	-256 / 0	0.04 (1)
B-C	-376 / 0	-78.0	-78.0	0.08 (1)	6.25	B-F	0 / 230	0.05 (1)
C-D	-8 / 0	-78.0	-78.0	0.09 (1)	10.00	F-C	0 / 76	0.02 (4)
E-D	-102 / 0	0.0	0.0	0.02 (1)	7.81	C-E	-375 / 0	0.08 (1)
I-H	0 / 0	-96.5	-96.5	0.16 (1)	10.00			
H-G	-1 / 0	-18.5	-18.5	0.16 (1)	10.00			
G-F	0 / 206	-18.5	-18.5	0.04 (1)	10.00			
F-E	0 / 350	-18.5	-18.5	0.09 (1)	10.00			

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



SPECIFIED LOADS:			
TOP CH.	LL	=	21.0 PSF
	DL	=	6.0 PSF
BOT CH.	LL	=	0.0 PSF
	DL	=	7.4 PSF
TOTAL LOAD		=	34.4 PSF

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT
OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= $L/120$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.01")
ALLOWABLE DEFL.(TL)= $L/120$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/681$ (0.03")

CSI: TC=0.09/1.00 (C-D:1), BC=0.16/1.00 (G-H:1)
WB=0.08/1.00 (C-E:1), SSI=0.12/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

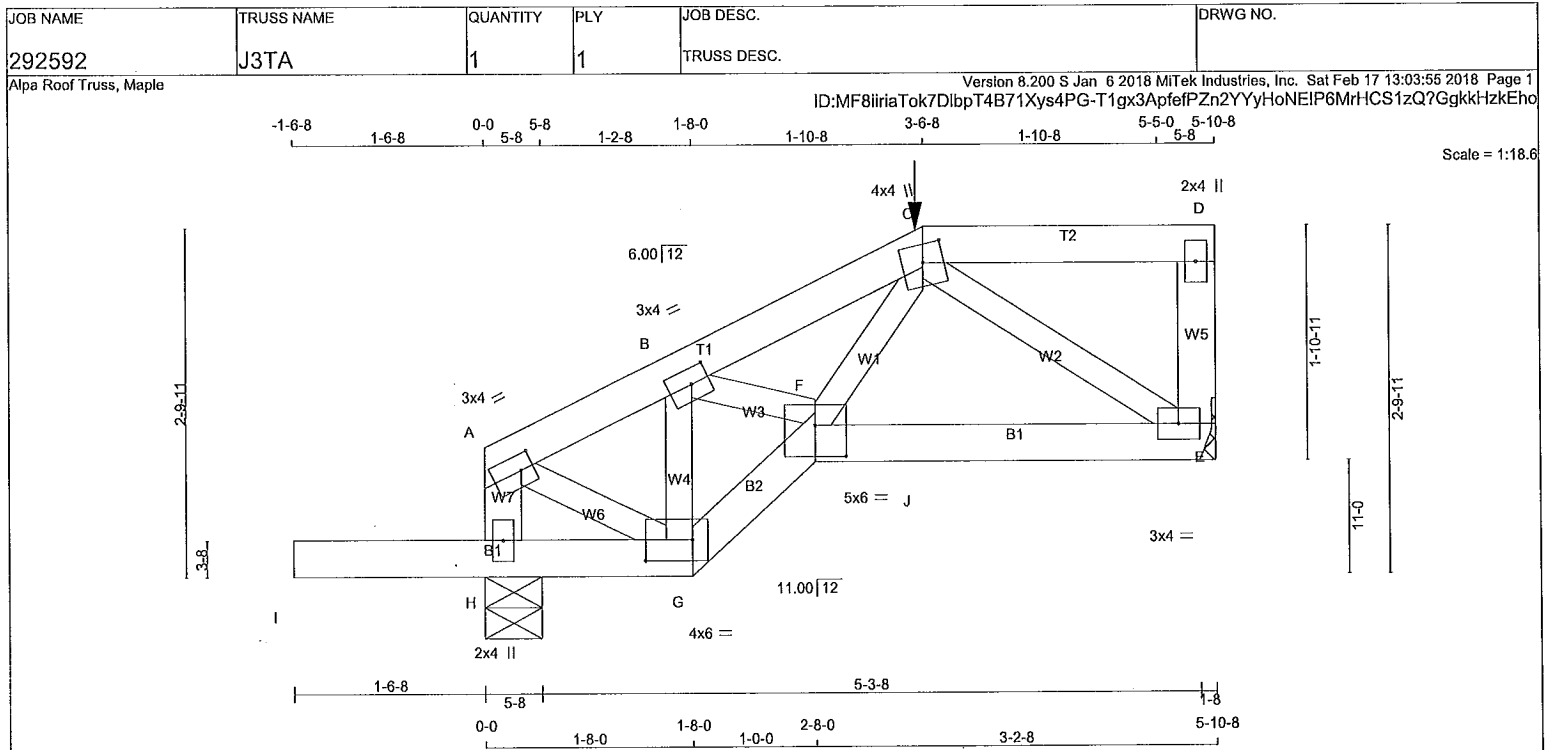
PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.53 (E) (INPUT = 0.90)
JSI METAL= 0.13 (E) (INPUT = 1.00)

A-18023481



LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.	
A - C	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	
H - A	2x4	DRY	No.2	SPF	
I - G	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
F - E	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-I	MT20	3.0	4.0	1.50	1.25
B	TMVW-I	MT20	3.0	4.0	1.50	1.75
C	TTWW+m	MT20	4.0	4.0	1.75	2.00
D	TMV+p	MT20	2.0	4.0		
E	BMVW1-I	MT20	3.0	4.0		
F	BBWW-I	MT20	5.0	6.0	3.00	3.00
G	BBWW-I	MT20	4.0	6.0	2.00	4.50
H	BMV1+p	MT20	2.0	4.0		

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 94.0 lbs FACTORED DOWN AT 3-6-8 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT REQD BRG	
JT		VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX
E	398	0		398	0		
H	524	0		524	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN	COMPONENT REACTIONS	WIND	DEAD	SOIL
E	283	176 / 0	0 / 0	0 / 0	107 / 0	0 / 0
H	374	224 / 0	0 / 0	0 / 0	150 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX (PLF)	MAX. CS1 (LC)	MEMB.	FORCE (LBS)	MAX. CS1 (LC)
FR-TO		FROM	TO	LENGTH	FR-TO		
A-B	-280 / 0	-78.0	-78.0	0.03 (1)	6.25	G-B	-354 / 0
B-C	-490 / 0	-78.0	-78.0	0.05 (1)	6.25	B-F	0 / 229
C-D	0 / 0	-108.0	-108.0	0.11 (1)	10.00	F-C	0 / 187
E-D	-126 / 0	0.0	0.0	0.01 (1)	7.81	C-E	-414 / 0
H-A	-285 / 0	0.0	0.0	0.03 (1)	7.81	A-G	0 / 277
I-H	0 / 0	-96.5	-96.5	0.17 (1)	10.00		
H-G	-1 / 0	-25.6	-25.6	0.17 (1)	10.00		
G-F	0 / 312	-25.6	-25.6	0.06 (1)	10.00		
F-J	0 / 342	-25.6	-25.6	0.12 (4)	10.00		
J-E	0 / 342	-25.6	-25.6	0.12 (4)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	3-6-8	-94	-94		FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHlp
LEFT SETBACK = 3-6-8
RIGHT SETBACK = 0-0
END SETBACK = 3-6-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADD'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL) = L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/704 (0.03")

CSI: TC=0.11/1.00 (C-D:1), BC=0.17/1.00 (G-H:1), WB=0.08/1.00 (C-E:1), SSI=0.13/1.00 (H-I:1)

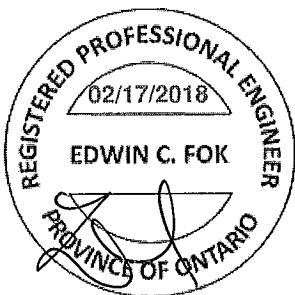
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

A-18023482

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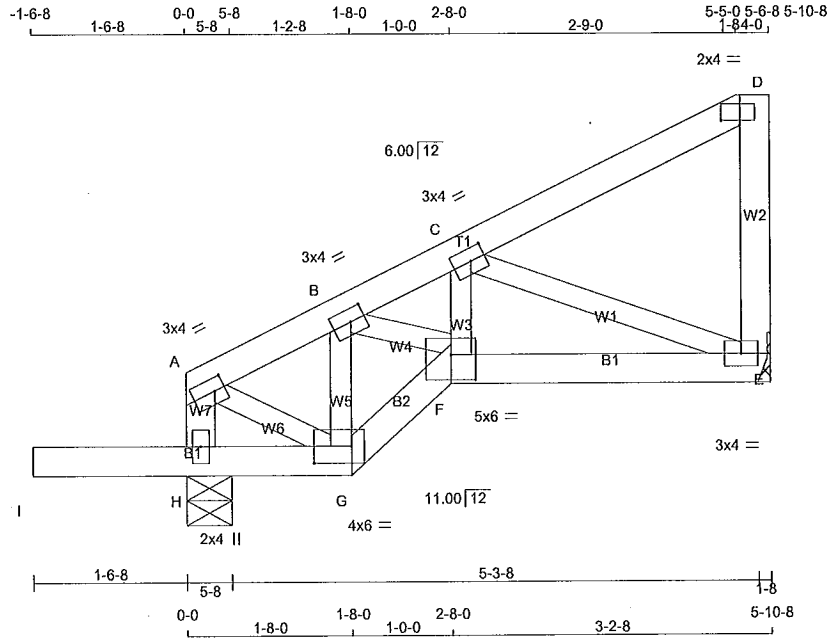
NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292592	J3TB	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Sat Feb 17 13:08:32 2018 Page 1

ID:MF8IiriaTok7DlbpT4B71Xys4PG-skVfBu9PPmJGWdDnhzGLzjNSIWqFsJcBisGuzkzkEdT



TOTAL WEIGHT = 26 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
E - D	2x4	DRY	No.2
H - A	2x4	DRY	No.2
I - G	2x4	DRY	No.2
G - F	2x4	DRY	No.2
F - E	2x4	DRY	No.2

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMVW-I	MT20	3.0	4.0	1.50 1.25
B	TMVW-I	MT20	3.0	4.0	1.50 1.75
C	TMVW-I	MT20	3.0	4.0	1.50 1.75
D	TVM-p	MT20	2.0	4.0	0.50 2.25
E	BMVW1-I	MT20	3.0	4.0	
F	BBWW-I	MT20	5.0	6.0	3.00 3.00
G	BBWW-I	MT20	4.0	6.0	2.00 4.50
H	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX	
E	264	0	264	0	0	0	
H	452	0	452	0	0	0	

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN	COMPONENT REACTIONS	WIND	DEAD	SOIL
E	188	115 / 0	0 / 0	0 / 0	73 / 0	0 / 0
H	322	197 / 0	0 / 0	0 / 0	125 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS	
MEMB.	FORCE (LBS)	VERT. LOAD	MAX. FACTORED	MEMB.	FORCE (LBS)
FR-TO		FROM	TO	FR-TO	
A-B	-191 / 0	-78.0	-78.0 0.03 (1)	G-B	-258 / 0
B-C	-376 / 0	-78.0	-78.0 0.08 (1)	B-F	0 / 230
C-D	-8 / 0	-78.0	-78.0 0.09 (1)	F-C	0 / 76
E-D	-102 / 0	0.0	0.0 0.02 (1)	C-E	-375 / 0
H-A	-219 / 0	0.0	0.0 0.02 (1)	A-G	0 / 184
I-H	0 / 0	-96.5	-96.5 0.16 (1)		
H-G	-1 / 0	-18.5	-18.5 0.16 (1)		
G-F	0 / 206	-18.5	-18.5 0.04 (1)		
F-E	0 / 350	-18.5	-18.5 0.09 (1)		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0 PSF
	DL	=	6.0 PSF
BOT CH.	LL	=	0.0 PSF
	DL	=	7.4 PSF
TOTAL LOAD = 34.4 PSF			

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)=	L/360 (0.20")
CALCULATED VERT. DEFL.(LL)=	L/999 (0.00")
ALLOWABLE DEFL.(TL)=	L/360 (0.20")
CALCULATED VERT. DEFL.(TL)=	L/999 (0.01")

CANTILEVER DEFLECTION:

ALLOWABLE DEFL.(LL)=	L/120 (0.19")
CALCULATED VERT. DEFL.(LL)=	L/999 (0.01")
ALLOWABLE DEFL.(TL)=	L/120 (0.19")
CALCULATED VERT. DEFL.(TL)=	L/681 (0.03")

CSI: TC=0.09/1.00 (C-D:1), BC=0.16/1.00 (G-H:1), WB=0.08/1.00 (C-E:1), SI=0.12/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 I.S BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

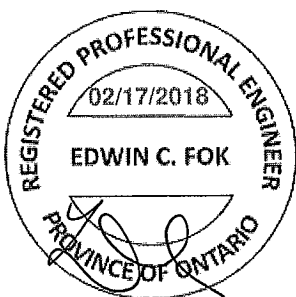
PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
618	354	1667
788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

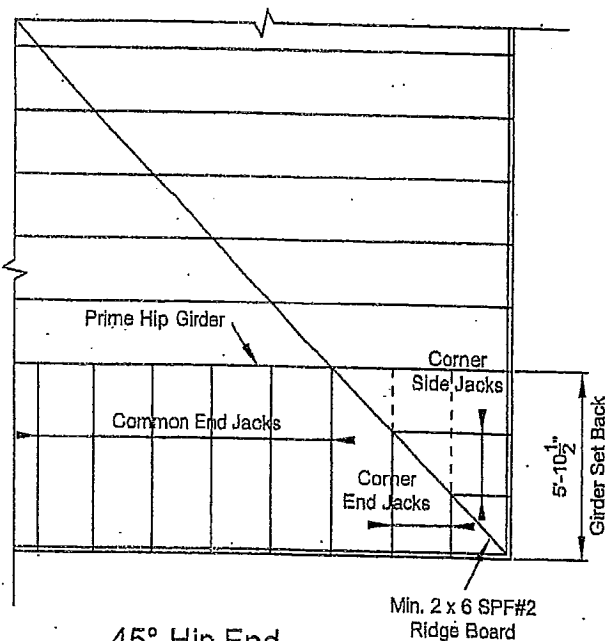
JSI GRIP= 0.53 (E) (INPUT = 0.90)
JSI METAL= 0.13 (E) (INPUT = 1.00)

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



A-18023483

STRACON ENGINEERING INC.



45° Hip End

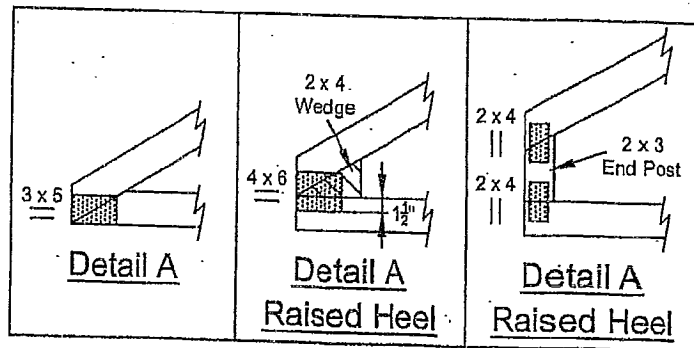
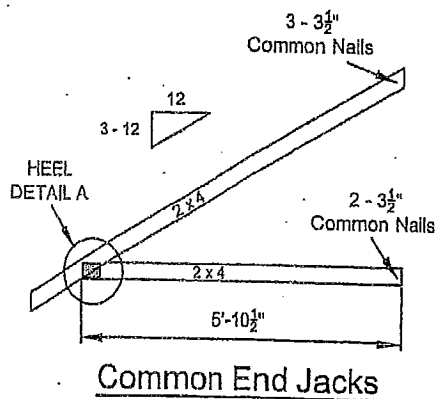
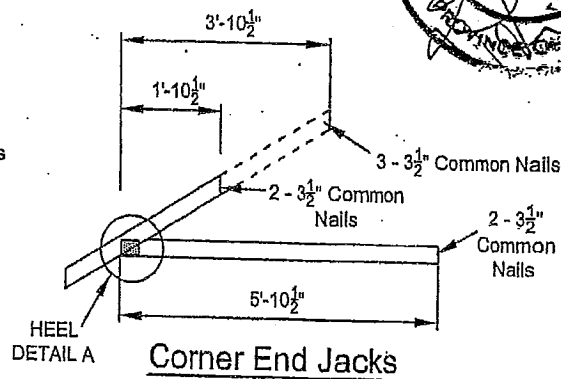
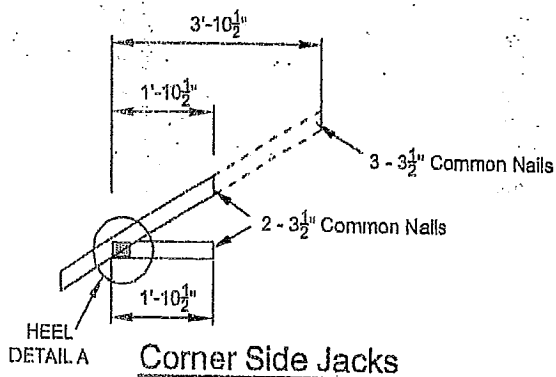
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD : 50.5 P.S.F



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

CS-51008

LUS – Double Shear Joist Hangers

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

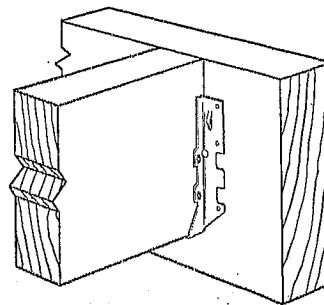
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

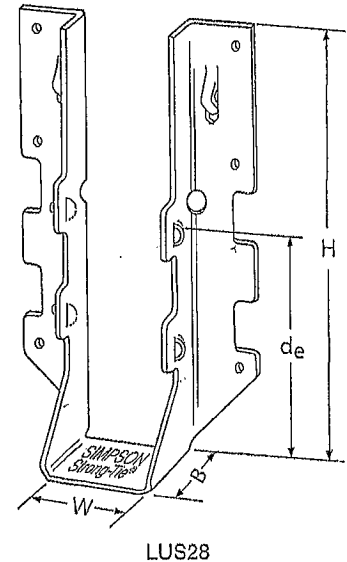
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified

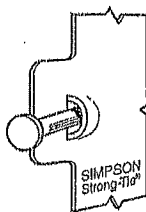


Typical LUS Installation



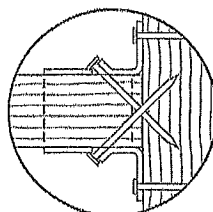
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _p =1.15)	Normal (K _p =1.00)	Uplift (K _p =1.15)	Normal (K _p =1.00)
LUS24	18	1½	3½	1¼	1 11/16	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3½	3½	2	1 11/16	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1½	4¾	1¼	3¾	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3½	4¾	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4¾	4¾	2	3¼	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1½	6¾	1¼	3¾	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3½	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4¾	6¼	2	3¼	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1½	7 11/16	1¼	3¾	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3½	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4¾	8 11/16	2	5¼	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.

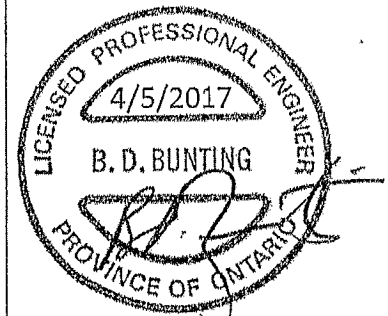


Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Top View.



LIMIT
STATES
DESIGN

This technical bulletin is effective until June 30, 2019, and reflects information available as of March 1, 2017. This information is updated periodically and should not be relied upon after June 30, 2019. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPEC LUS17-3/17 exp. 6/19

(800) 999-5099
strongtie.com

HUS/LJS – Double Shear Joist Hangers



All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

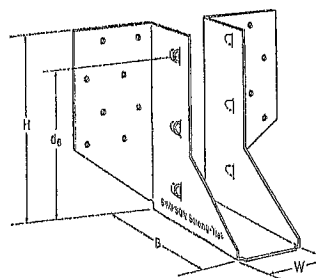
- Factored resistances are in accordance with CSA O86 -14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

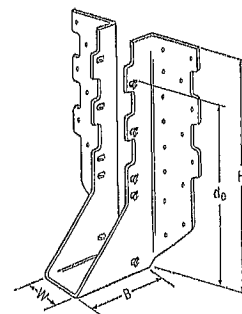
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

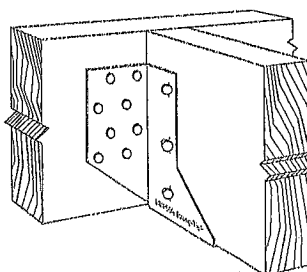
- See current catalogue for options



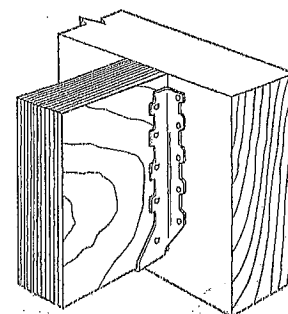
LJS26DS



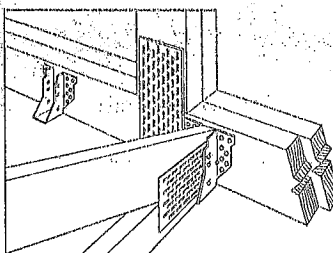
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



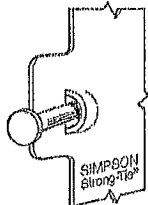
Typical HUS
Installation



Typical HUS Installation
(Truss Designer to provide fastener
quantity for connecting multiple
members together)

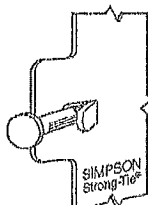
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d ₀ ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _D =1.15)	Normal (K _D =1.00)	Uplift (K _D =1.15)	Normal (K _D =1.00)
LJS26DS	18	1⅞	5	3½	4⅞	(16) 16d	(6) 16d	2065	4265	1460	4115
HUS26	16	1⅞	5⅞	3	3⅞	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1⅞	7⅞	3	6⅞	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1⅞	9⅞	3	7⅞	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1⅞	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

1. d₀ is the distance from the seat of the hanger to the highest joist nail.

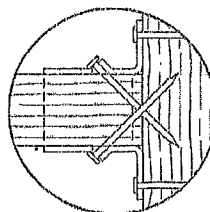


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

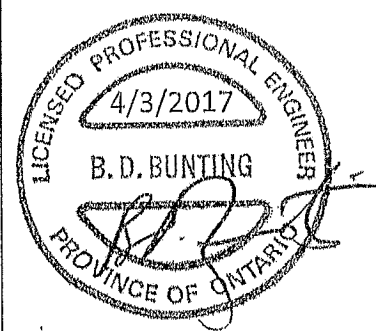
U.S. Patent
5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT
STATES
DESIGN

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T-SPECHUS17 3/17 exp. 6/19

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