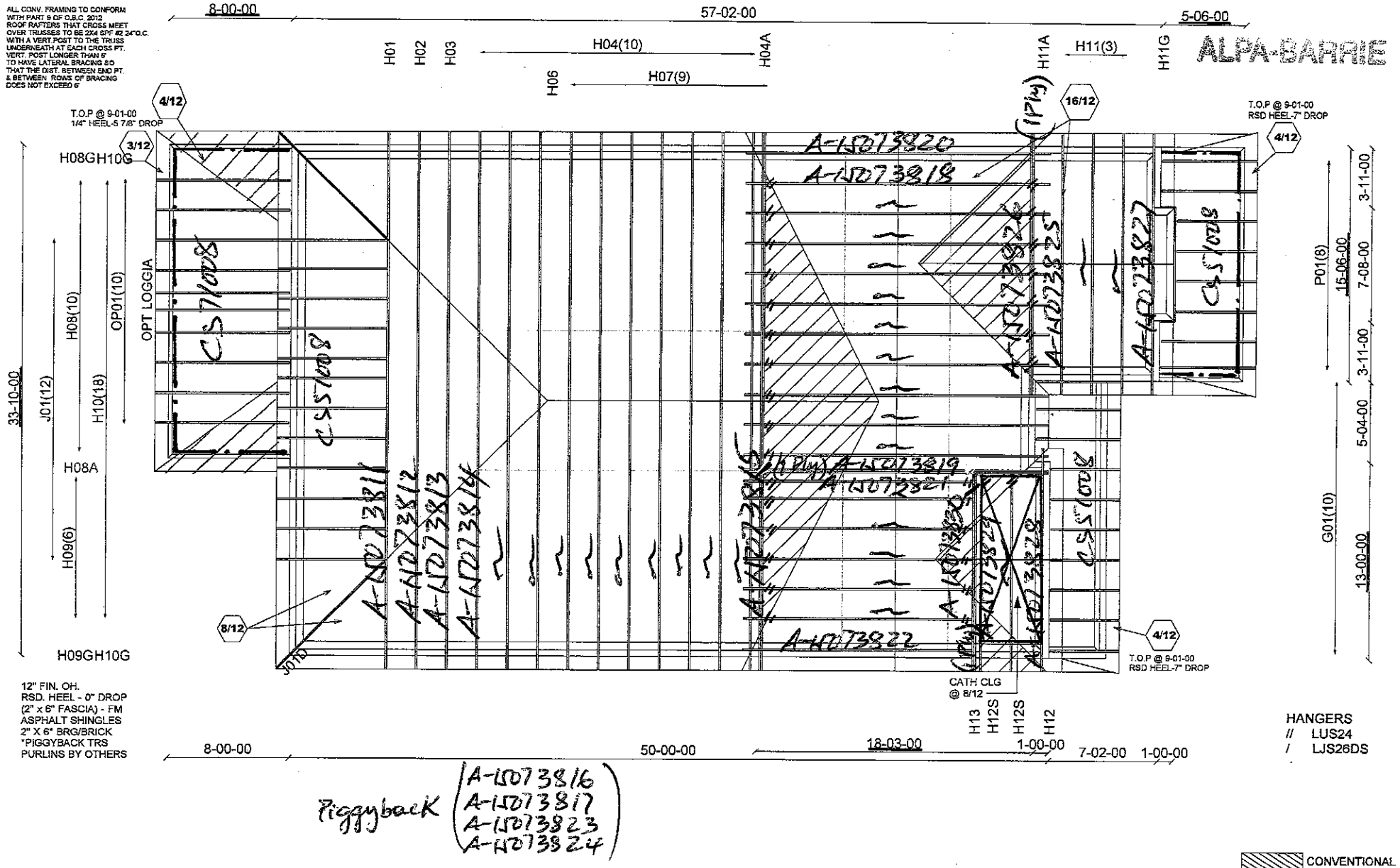
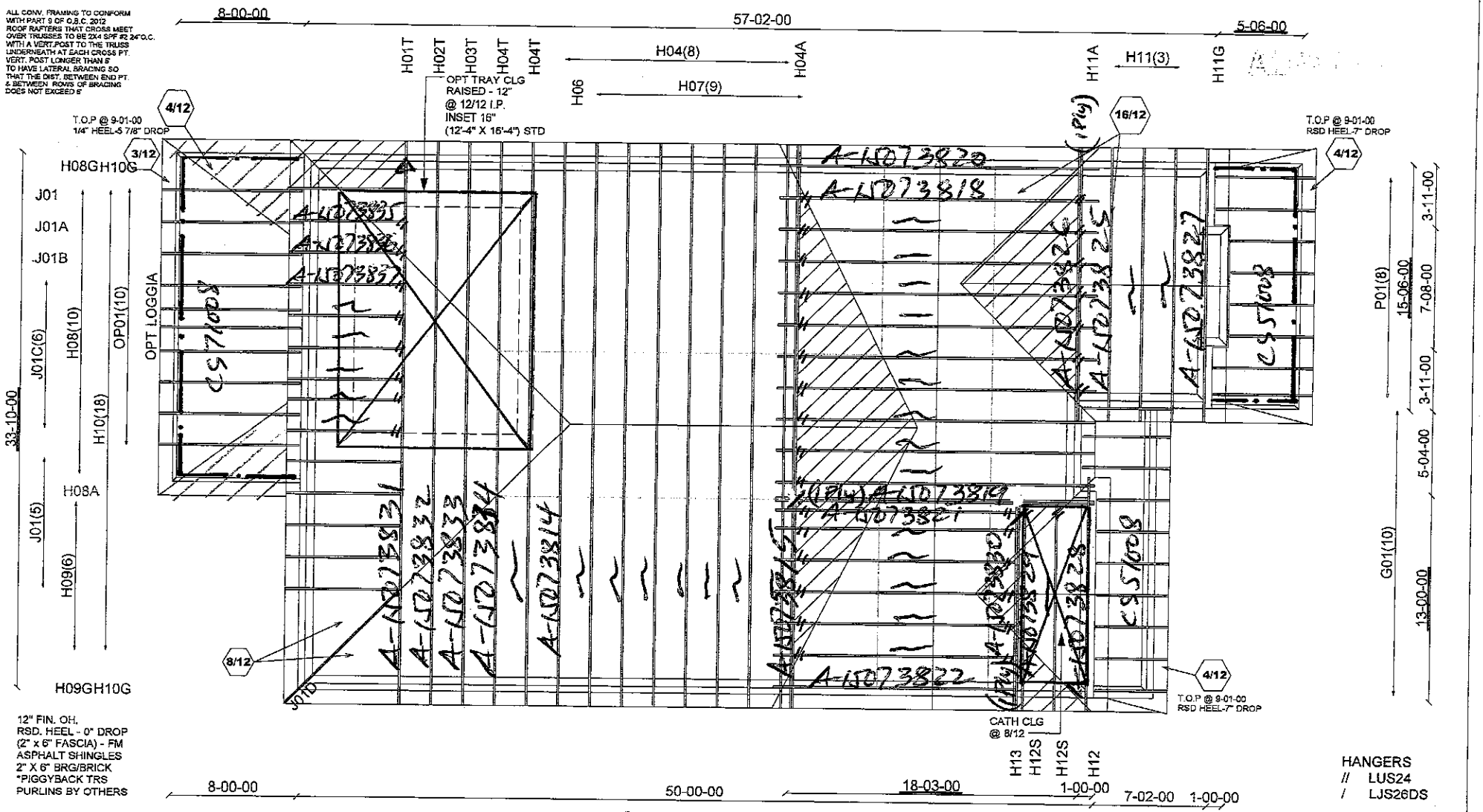


Mitek Ver: 7.5.0

Job Track: 40297	Builder / Location: GOLD PARK / KLEINBURG	Model / Elevation: 42-3 / A	STD OR OPT 2ND FLR
Layout ID: 253629			
Plan Log: 79711	Project: HUNTINGTON & NASHVILLE		
	Date: 4/30/2015 Designer: cheryle		

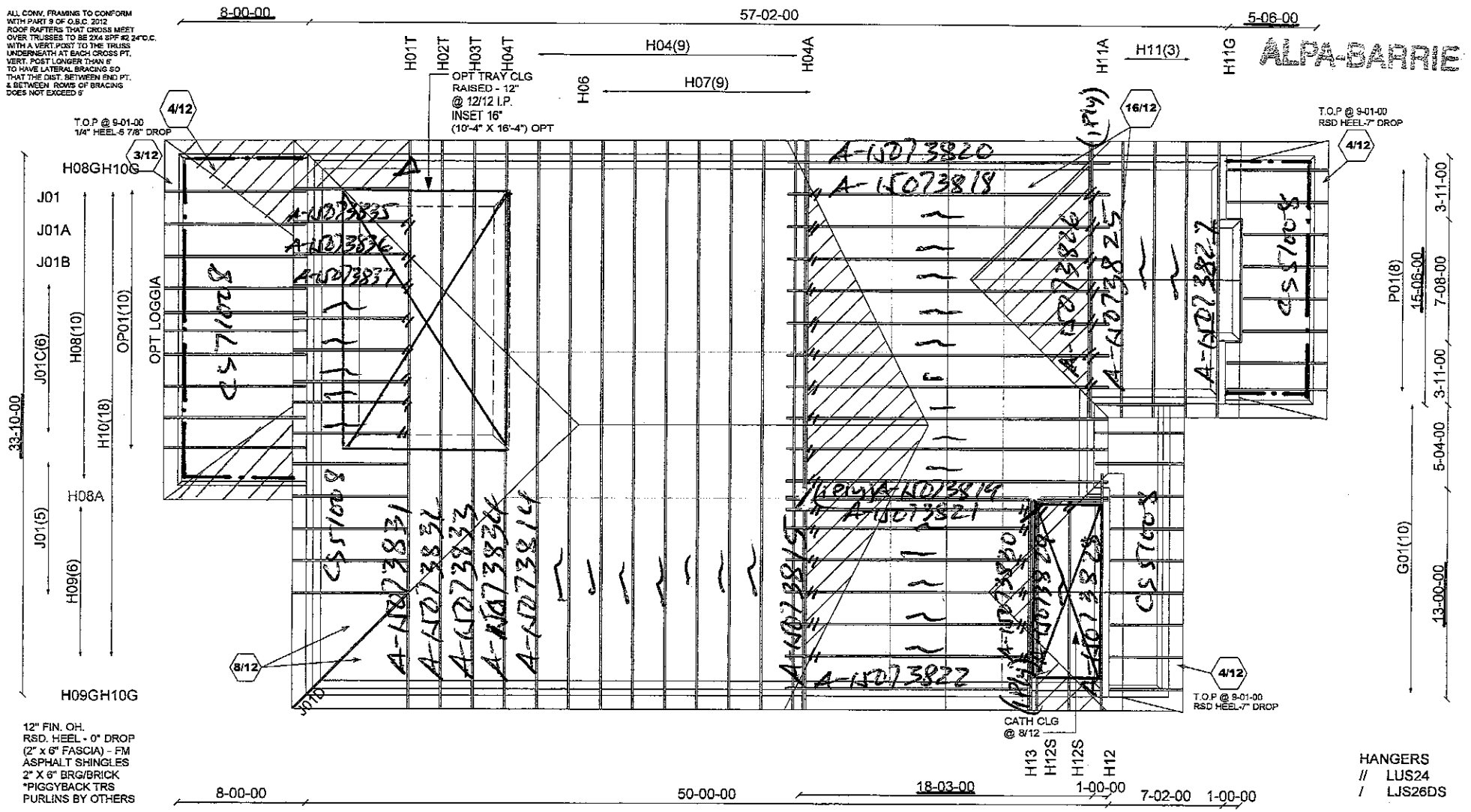


Piggyback (A-15073816, A-15073817, A-15073823, A-15073824)

CONVENTIONAL FRAMING BY OTHERS

Mitek Ver: 7.5.0

Job Track: 40297	Builder / Location: GOLD PARK / KLEINBURG	Model / Elevation: 42-3	/ A OPT TRAY
Layout ID: 253630	Project: HUNTINGTON & NASHVILLE		
Plan Log: 79711	Date: 4/30/2015 Designer: cheryle		

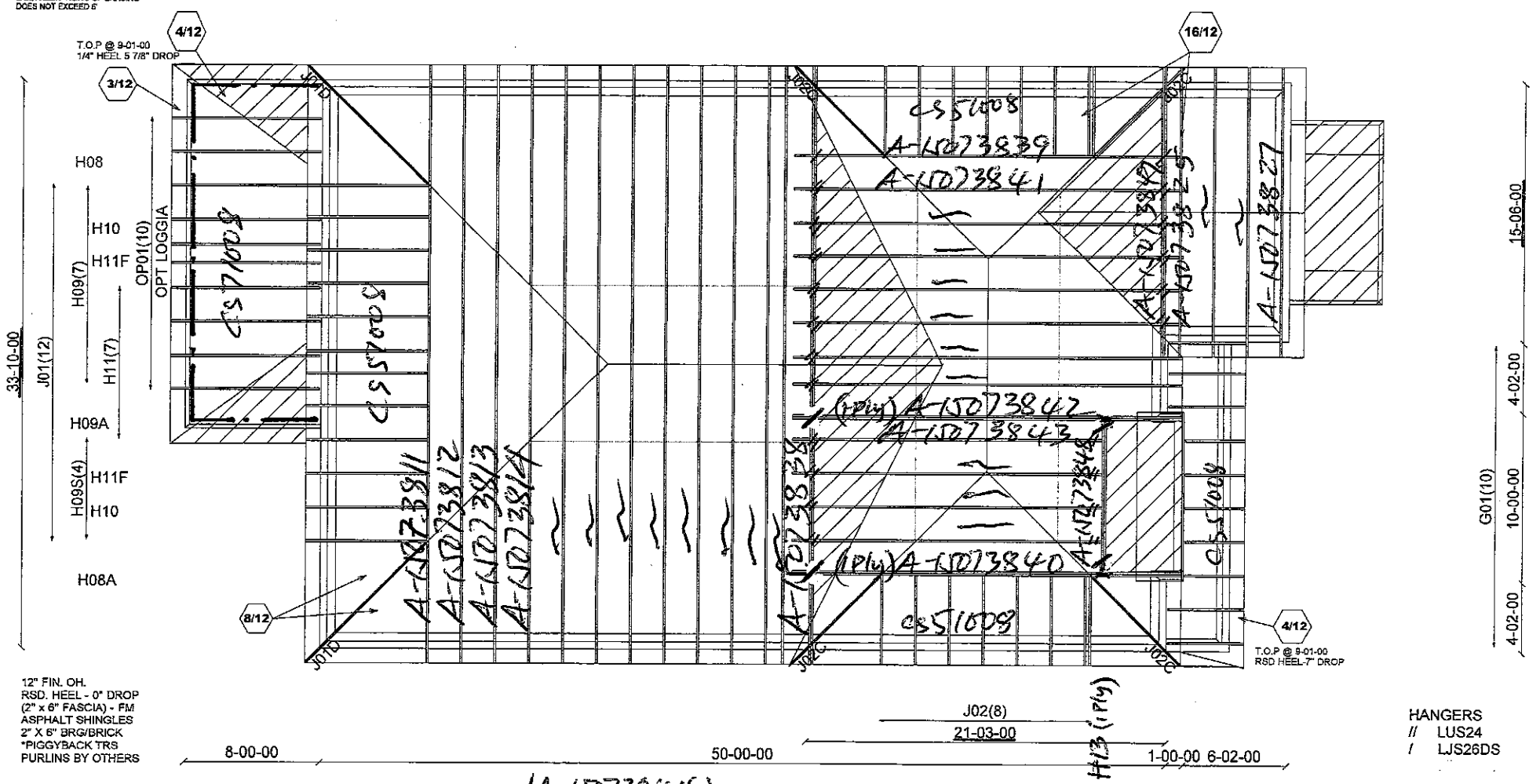


Mitek Ver: 7.5.0

Job Track: 40297	Builder / Location: GOLD PARK / KLEINBURG	Model / Elevation: 42-3	/ A OPT 2ND W/TRAY
Layout ID: 253631			
Plan Log: 79711	Project: HUNTINGTON & NASHVILLE		
	Date: 4/30/2015 Designer: cheryle		

ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. 2012.
ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2X4 S/P/F #2 24"O.C. WITH A VERT.POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DIST. BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

ALPA-BARRIE



12" FIN. OH.
RSD. HEEL - 0" DROP
(2" x 6" FASCIA) - FM
ASPHALT SHINGLES
2" X 6" BRG/BRICK
*PIGGYBACK TRS
PURLINS BY OTHERS

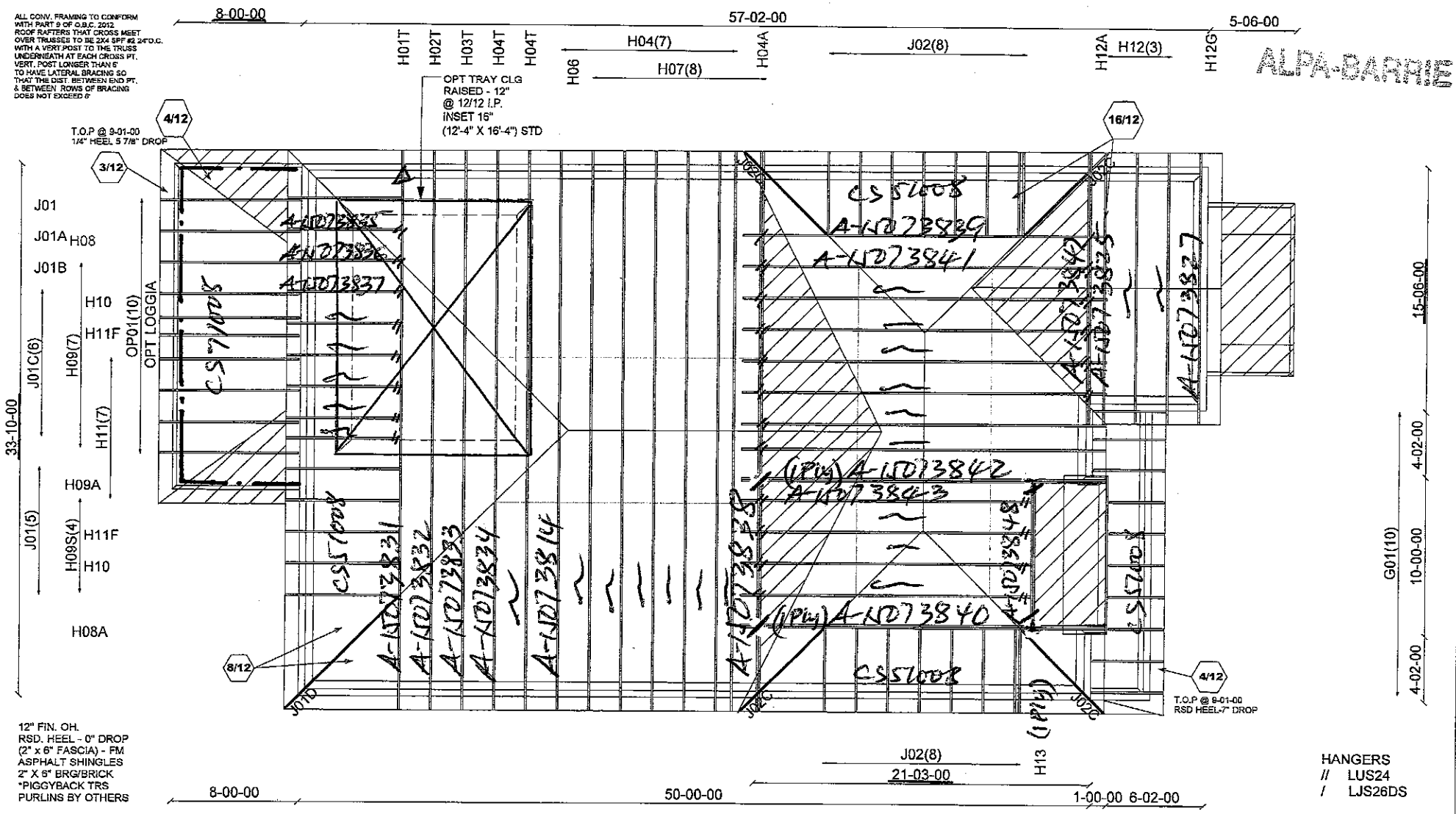
HANGERS
// LUS24
/ LJS26DS

Piggyback (A-150738 44, A-150738 45, A-150738 46, A-150738 16, A-150738 17)

CONVENTIONAL FRAMING BY OTHERS

Mitek Ver: 7.5.0

Job Track: 40297	Builder / Location: GOLD PARK / KLEINBURG	Model / Elevation: 42-3 / B	STD OR OPT 2ND FLR.
Layout ID: 253632			
Plan Log: 79711	Project: HUNTINGTON & NASHVILLE		
	Date: 4/30/2015 Designer: cheryle		

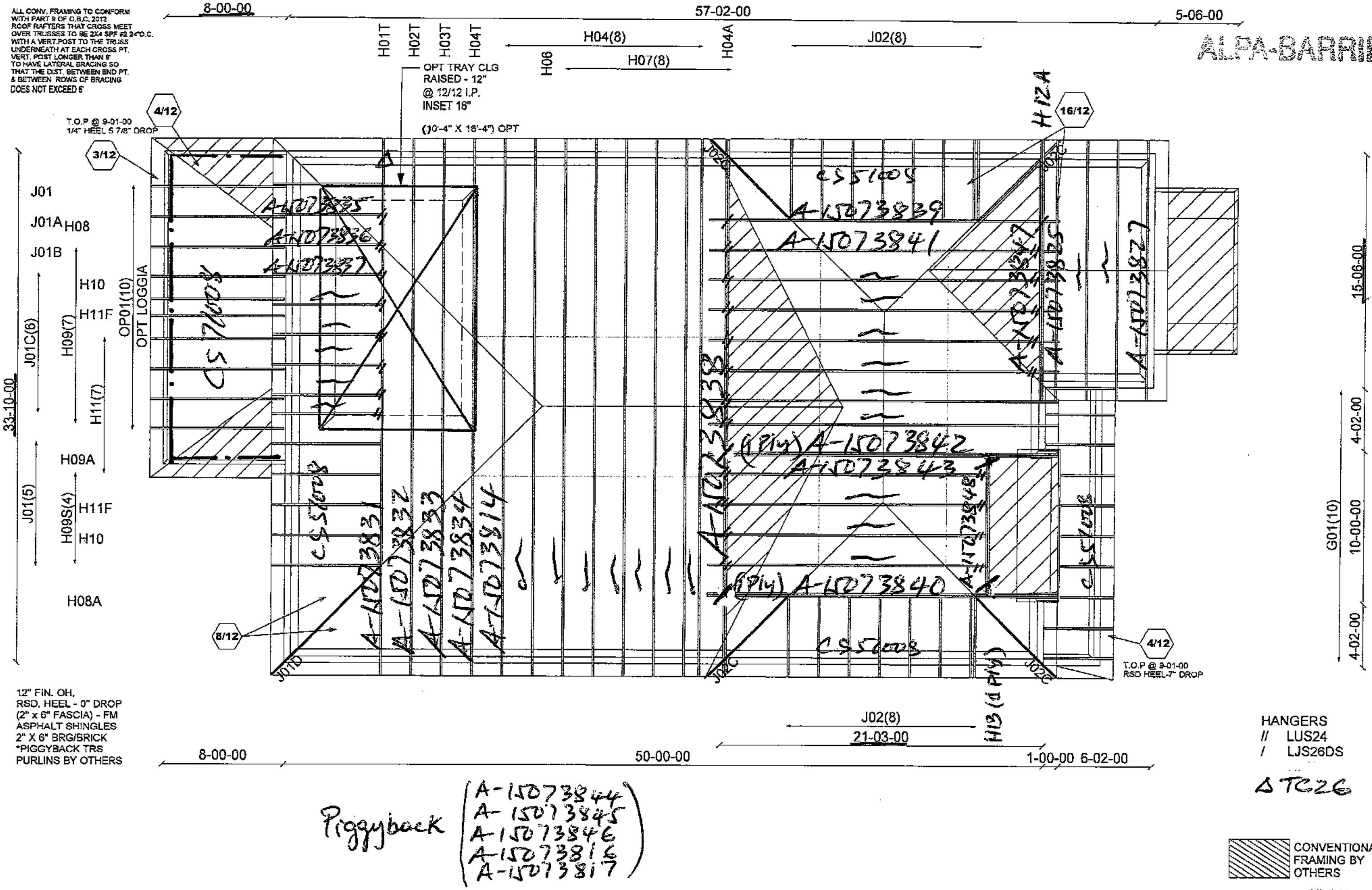


Mitek Ver: 7.5.0

Job Track: 40297	Builder / Location: GOLD PARK / KLEINBURG	Model / Elevation: 42-3	/ B OPT TRAY
Layout ID: 253633	Project: HUNTINGTON & NASHVILLE		
Plan Log: 79711	Date: 4/30/2015 Designer: cheryle		

ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. 2012 ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2X4 SPF #2 24"O.C. WITH A VERT.POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DIST. BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

ALPA-BARRIE



12" FIN. OH.
RSD. HEEL - 0" DROP
(2" x 6" FASCIA) - FM
ASPHALT SHINGLES
2" X 6" BRG/BRICK
*PIGGYBACK TRS
PURLINS BY OTHERS

HANGERS
// LUS24
/ LJS26DS

△ TC26

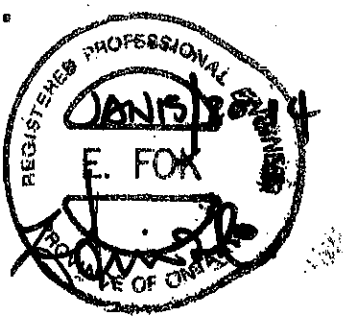
CONVENTIONAL FRAMING BY OTHERS

Mitek Ver: 7.5.0

Job Track: 40297	Builder / Location: GOLD PARK / KLEINBURG	Model / Elevation: 42-3 / B OPT 2ND W/TRAY
Layout ID: 253634	Project: HUNTINGTON & NASHVILLE	
Plan Log: 79711	Date: 4/30/2015 Designer: cheryle	

Stracon Engineering Inc.

69 Graydon Crescent
Richmond Hill, Ontario
L4B 3W7
(905) 832-2250 Fax (905) 832-0286



RESPONSIBILITIES

1. Stracon Engineering Inc. is responsible for the design of trusses as individual components.
2. It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities with jurisdictions.
3. All dimensions are to be verified by owner contractor, architect or other authority before manufacture.
4. Stracon Engineering Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Stracon Engineering Inc. drawings is specified for the truss as a component only and forms an integral part of the truss design.
5. It is the truss manufacture's responsibility to ensure that trusses are manufactured in conformance with Stracon Engineering Inc. specifications outlined below.

SPECIFICATIONS

1. Trusses designed by Stracon Engineering Inc. conforms to the relevant section of the Ontario Building Code of Canada (Part 9 or Part 4) or to the Canadian code for farm buildings, whichever applies to the building type indicated on the drawings as well as to the procedures established by the truss plate institute of Canada. Unit stresses used are as per CSA-086-09.
2. Lumber is to be the sizes and grade specified.
3. Moisture content of lumber is not to exceed 19% in service unless otherwise specified.
4. Lumber not to be treated with chemicals unless otherwise specified.
5. Plates shall be applied to both faces of the truss at each joint and shall be positioned as specified.
6. The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals not exceeding 12.5 times in thickness.
7. Where not rigid ceiling is attached directly to the bottom chord, laterally brace the chords at intervals not exceeding 3M (10') o.c.

January 15, 2014

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253632	H13	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

ID:AyrJx9Y?fa2EXF7DRQxSq4zMEz1-dgCHTq2KNFetA?EkhSkWfDNQnMDnrEtV?c0McQyxBg?

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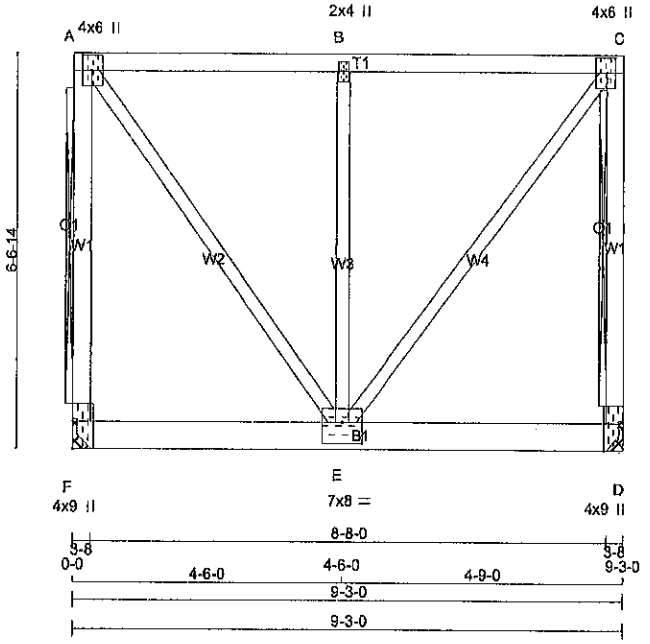

TOTAL WEIGHT = 67 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	3.00	1.75
B	TMW+w	MT20	2.0	4.0		
C	TMVW+p	MT20	4.0	6.0	3.00	1.75
D	BMV1+t	MT20	4.0	9.0	Edge	0.50
E	BMWWW-t	MT20	7.0	8.0	4.25	4.00
F	BMV1+t	MT20	4.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	2195	0	2195	0	0	IN-SX	IN-SX
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-8	
D	2195	0	2195	0	0	HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1546	1047 / 0	0 / 0	0 / 0	0 / 0	499 / 0	0 / 0
D	1546	1047 / 0	0 / 0	0 / 0	0 / 0	499 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.55FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

2x4 DRY SPF No.2 T-BRACE REQUIRED AT A-F, C-D
FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- A	-1571 / 0	0.0	0.0	0.37 (1)	7.81	A- E	0 / 1696	0.42 (1)
A- B	-996 / 0	-115.9	-115.9	0.45 (1)	5.55	E- B	-557 / 0	0.44 (1)
B- C	-996 / 0	-115.9	-115.9	0.45 (1)	5.55	E- C	0 / 1638	0.41 (1)
D- C	-1517 / 0	0.0	0.0	0.36 (1)	7.81			
F- E	0 / 0	-358.6	-358.6	0.61 (1)	10.00			
E- D	0 / 0	-358.6	-358.6	0.61 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 0-0
END SETBACK = 4-7-0
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 17-0-0
END DISTANCE = 9-0-0
END SPAN CARRIED = 17-0-0
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.31")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.31")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.45 (B-C:1) , BC=0.61 (D-E:1) , WB=0.44 (B-E:1) , SSI=0.68 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

LICENSED PROFESSIONAL ENGINEER
07/17/2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

A-NO73848

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253632	H12A	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MiTek Industries, Inc. Fri Jul 17 11:13:01 2015 Page 1
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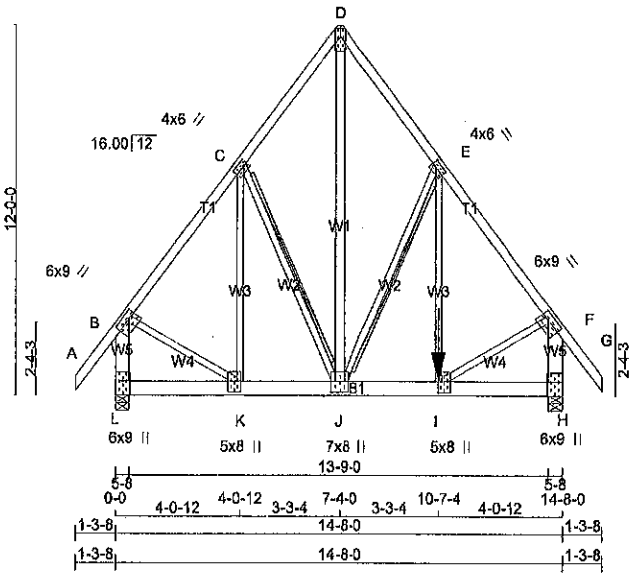

TOTAL WEIGHT = 110 lb
(M)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
L - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
J - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	6.0	9.0	2.25	3.00
C	TMWW-t	MT20	4.0	6.0	2.00	1.75
D	TTW+p	MT20	4.0	9.0	3.25	Edge
E	TMWW-t	MT20	4.0	6.0	2.00	1.75
F	TMVW-t	MT20	6.0	9.0	2.25	3.00
H	BMV1+t	MT20	6.0	9.0	Edge	0.50
I	BMWW+t	MT20	5.0	8.0	4.25	1.50
J	BMWWW+t	MT20	7.0	8.0	4.25	3.50
K	BMWW+t	MT20	5.0	8.0	4.25	1.50
L	BMV1+t	MT20	6.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1419.3 lbs FACTORED DOWN AT 10-7-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	4637	0	4637	0	0	5-8	5-8
H	3816	0	3816	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	3265	2225 / 0	0 / 0	0 / 0	0 / 0	1040 / 0	0 / 0
H	2666	1832 / 0	0 / 0	0 / 0	0 / 0	854 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.57FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

2x4 DRY SPF No.2 T-BRACE REQUIRED AT E-J, C-J
FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-70.5	-70.5	0.11 (1)	10.00	J- D	0 / 3851	0.68 (1)
B- C	-3215 / 0	-70.5	-70.5	0.47 (1)	3.60	J- E	-1115 / 0	0.55 (1)
C- D	-2540 / 0	-70.5	-70.5	0.26 (1)	4.11	I- E	0 / 1323	0.33 (1)
D- E	-2540 / 0	-70.5	-70.5	0.26 (1)	4.11	C- J	-1043 / 0	0.51 (1)
E- F	-3264 / 0	-70.5	-70.5	0.48 (1)	3.67	K- C	0 / 1218	0.30 (1)
F- G	0 / 40	-70.5	-70.5	0.11 (1)	10.00	B- K	0 / 2136	0.53 (1)
L- B	-3703 / 0	0.0	0.0	0.33 (1)	5.51	I- F	0 / 2168	0.54 (1)
H- F	-3766 / 0	0.0	0.0	0.33 (1)	5.47			
L- K	0 / 0	-532.0	-532.0	0.60 (1)	10.00			
K- J	0 / 1939	-532.0	-532.0	0.69 (1)	10.00			
J- I	0 / 1968	-532.0	-532.0	0.65 (1)	10.00			
I- H	0 / 0	-61.5	-17.5	0.05 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
I	10-7-4	-1419	-1419	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder
START DISTANCE = 10-7-4
START SPAN CARRIED = 4-0-0
END DISTANCE = 14-8-0
END SPAN CARRIED = 2-0-0
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.49")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.49")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.48 (E-F:1) , BC=0.69 (J-K:1) , WB=0.68 (D-J:1) , SSI=0.81 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL.. = 0.250 Inches

PLATE ROTATION TOL. = 90 deg.

A 207847

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253632	H11F	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

ID:AyrJx9Y?fa2EXF7DRQxSq4zMEz1-hH4X2914rdO9xh4MZ1i2aoH9LZcHNQJCXIXFYYyxBg

5x6 II

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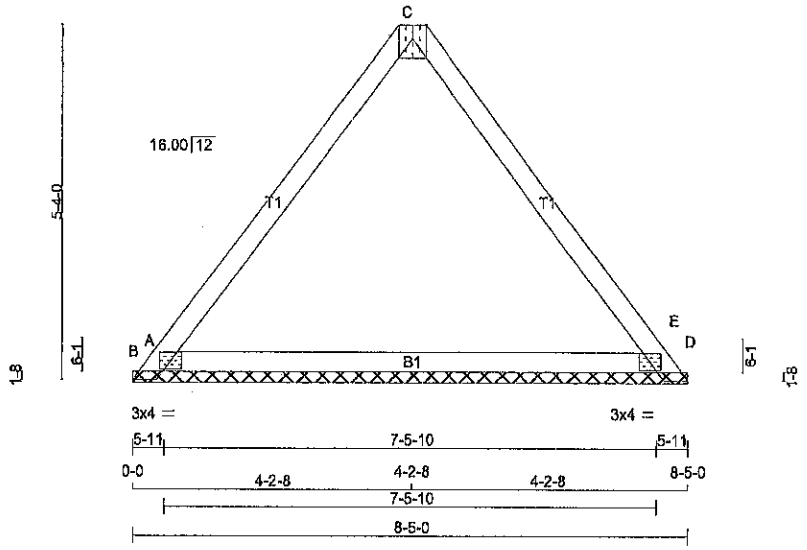

TOTAL WEIGHT = 2 X 24 = 48 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TMTM+p	MT20	6.0	6.0	Edge	
D	TMB1-I	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
A	-308	0	0	0	-308	8-5-0 (7-5-10)-8	
E	-308	0	0	0	-308	8-5-0 (7-5-10)-8	
B	667	0	667	0	0	8-5-0 (7-5-10)-8	
D	667	0	667	0	0	8-5-0 (7-5-10)-8	

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): A , E

PROVIDE ANCHORAGE AT BEARING JOINT A FOR 308 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT E FOR 308 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	-209	0 / -185	0 / 0	0 / 0	0 / 0	0 / -24	0 / 0
E	-209	0 / -185	0 / 0	0 / 0	0 / 0	0 / -24	0 / 0
B	463	357 / 0	0 / 0	0 / 0	0 / 0	106 / 0	0 / 0
D	463	357 / 0	0 / 0	0 / 0	0 / 0	106 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, E, B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 228	-88.0	-88.0	0.19 (1)	10.00			
B-C	-150 / 0	-70.5	-70.5	0.16 (1)	6.25			
C-D	-150 / 0	-70.5	-70.5	0.16 (1)	6.25			
D-E	0 / 228	-88.0	-88.0	0.19 (1)	10.00			
B-D	0 / 74	-17.5	-17.5	0.29 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.19 (D-E:1) , BC=0.29 (B-D:4) , WB=0.00 (n/a:0) , SSI=0.19 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1666

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.70 (D) (INPUT = 0.90)
JSI METAL= 0.15 (D) (INPUT = 1.00)

A-N073846

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253632	H11	6	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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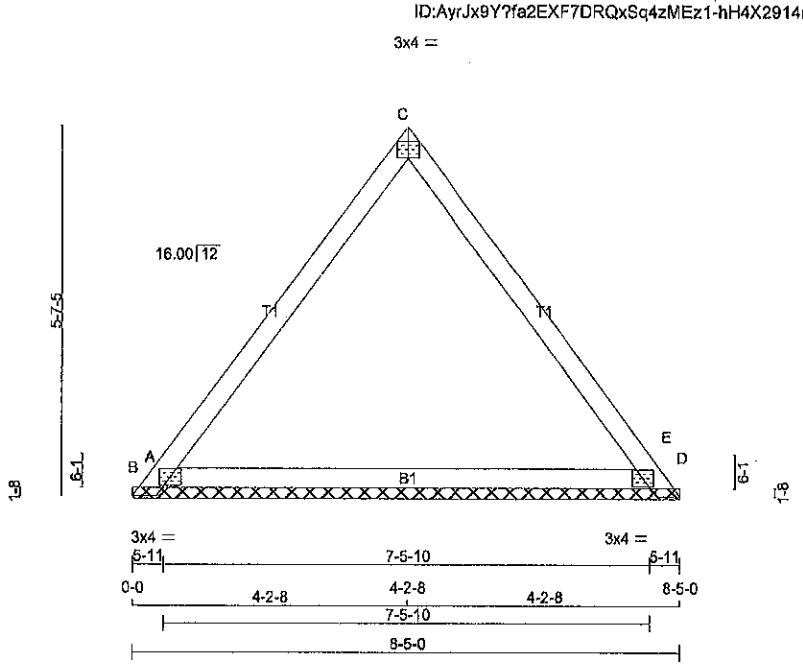

TOTAL WEIGHT = 6 X 24 = 147 lb
[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TT-p	MT20	3.0	4.0	3.25	Edge
D	TMB1-I	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
A	-308	0	0	0	-308	8-5-0 (7-5-10)	1-8
E	-308	0	0	0	-308	8-5-0 (7-5-10)	1-8
B	667	0	667	0	0	8-5-0 (7-5-10)	1-8
D	667	0	667	0	0	8-5-0 (7-5-10)	1-8

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): A , E

PROVIDE ANCHORAGE AT BEARING JOINT A FOR 308 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT E FOR 308 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	-209	0 / -185	0 / 0	0 / 0	0 / 0	0 / -24	0 / 0
E	-209	0 / -185	0 / 0	0 / 0	0 / 0	0 / -24	0 / 0
B	463	357 / 0	0 / 0	0 / 0	0 / 0	106 / 0	0 / 0
D	463	357 / 0	0 / 0	0 / 0	0 / 0	106 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, E, B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX UNBRAC LENGTH	MAX CSI (LC)
A-B	0 / 228	-88.0	-88.0	0.19	(1)			10.00	
B-C	-150 / 0	-70.5	-70.5	0.16	(1)			6.25	
C-D	-150 / 0	-70.5	-70.5	0.16	(1)			6.25	
D-E	0 / 228	-88.0	-88.0	0.19	(1)			10.00	
B-D	0 / 74	-17.5	-17.5	0.29	(4)			10.00	

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.19 (D-E:1) , BC=0.29 (B-D:4) , WB=0.00 (n/a:0) , SSI=0.19 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.70 (D) (INPUT = 0.90)
JSI METAL= 0.15 (D) (INPUT = 1.00)

A-1073845

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253632	H10	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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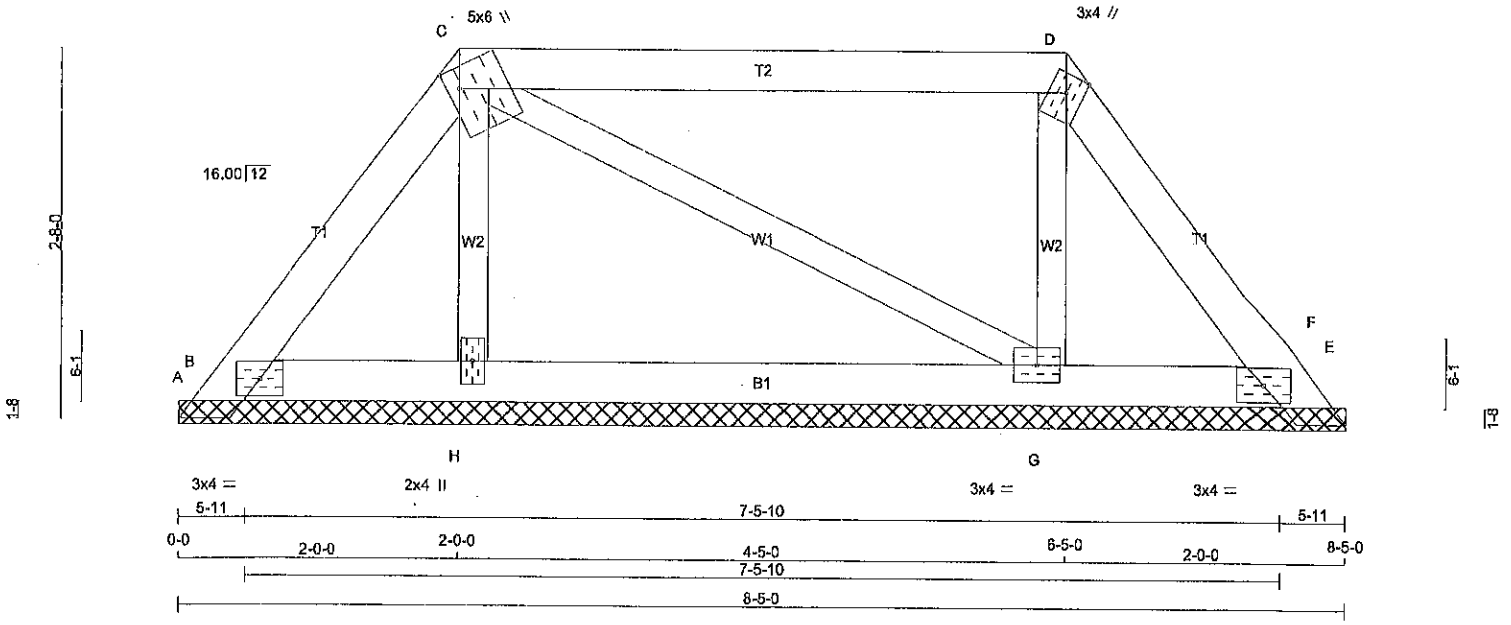

TOTAL WEIGHT = 2 X 28 = 56 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW+m	MT20	5.0	6.0	1.75	1.00
D	TTW+m	MT20	3.0	4.0	Edge	
E	TMB1-I	MT20	3.0	4.0		
G	BMWW1-t	MT20	3.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
A	-36	0	0	0	-36	8-5-0 (7-5-10)-8	
F	-37	0	0	0	-37	8-5-0 (7-5-10)-8	
B	199	0	199	0	0	8-5-0 (7-5-10)-8	
E	186	0	186	0	0	8-5-0 (7-5-10)-8	
H	191	0	191	0	0	8-5-0 (7-5-10)-8	
G	216	0	216	0	0	8-5-0 (7-5-10)-8	

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): A , F

PROVIDE ANCHORAGE AT BEARING JOINT A FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT F FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	-24	0 / -23	0 / 0	0 / 0	0 / 0	0 / -1	0 / 0
F	-25	0 / -24	0 / 0	0 / 0	0 / 0	0 / -1	0 / 0
B	136	116 / 0	0 / 0	0 / 0	0 / 0	20 / 0	0 / 0
E	127	109 / 0	0 / 0	0 / 0	0 / 0	19 / 0	0 / 0
H	137	75 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0
G	154	90 / 0	0 / 0	0 / 0	0 / 0	64 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, F, B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH
A-B	0 / 36	-88.0	-88.0	0.03 (1)	10.00
B-C	-90 / 0	-70.5	-70.5	0.02 (1)	6.25
C-D	-29 / 0	-70.5	-70.5	0.23 (1)	6.25
D-E	-73 / 0	-70.5	-70.5	0.03 (1)	6.25
E-F	0 / 37	-88.0	-88.0	0.03 (1)	10.00
B-H	0 / 46	-17.5	-17.5	0.05 (4)	10.00
H-G	0 / 40	-17.5	-17.5	0.05 (4)	10.00
G-E	0 / 38	-17.5	-17.5	0.05 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H-C	-126 / 0	0.02 (1)
C-G	-12 / 0	0.00 (1)
G-D	-146 / 0	0.02 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.23 (C-D:1) , BC=0.05 (B-H:4) , WB=0.02 (D-G:1) , SSI=0.12 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.38 (D) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

A-N0738444

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253632	H09S	4	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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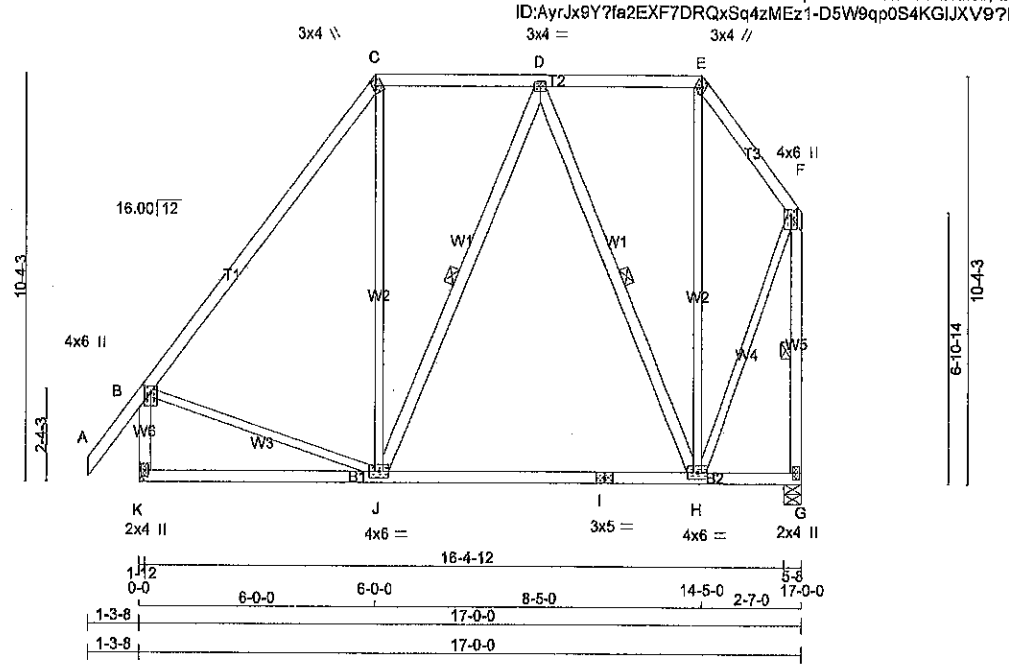

TOTAL WEIGHT = 4 X 109 = 436 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	1650F 1.5E	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
J - D	2x4	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TTW+m	MT20	3.0	4.0	1.75	1.00
D	TMWW-t	MT20	3.0	4.0		
E	TTW+m	MT20	3.0	4.0	1.75	1.00
F	TMVW+p	MT20	4.0	6.0	2.00	2.00
G	BMV1+p	MT20	2.0	4.0		
H	BMWWW-t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BMV1+p	MT20	2.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
K	872	0	872	0	0	IN-SX	IN-SX
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	
G	785	0	785	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	614	416 / 0	0 / 0	0 / 0	0 / 0	199 / 0	0 / 0
G	557	357 / 0	0 / 0	0 / 0	0 / 0	200 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF D-J, D-H, F-G.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO					FR-TO			
A- B	0 / 40	-70.5	-70.5	0.08 (1)	10.00	J- C	0 / 107	0.04 (4)
B- C	-527 / 0	-70.5	-70.5	0.28 (1)	6.25	J- D	-44 / 17	0.03 (1)
C- D	-315 / 0	-78.0	-78.0	0.18 (1)	6.25	D- H	-390 / 0	0.25 (1)
D- E	-185 / 0	-78.0	-78.0	0.18 (1)	6.25	H- E	0 / 60	0.02 (4)
E- F	-311 / 0	-70.5	-70.5	0.06 (1)	6.25	B- J	0 / 331	0.07 (1)
K- B	-834 / 0	0.0	0.0	0.11 (1)	7.81	H- F	0 / 485	0.11 (1)
G- F	-789 / 0	0.0	0.0	0.18 (1)	6.25			
K- J	0 / 0	-17.5	-17.5	0.23 (4)	10.00			
J- I	0 / 333	-17.5	-17.5	0.25 (4)	10.00			
I- H	0 / 333	-17.5	-17.5	0.25 (4)	10.00			
H- G	0 / 0	-17.5	-17.5	0.18 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
6.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.57")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.57")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.28 (B-C:1) , BC=0.25 (H-J:4) , WB=0.25 (D-H:1) , SSI=0.16 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.60

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (J) (INPUT = 0.90)
JSI METAL= 0.20 (I) (INPUT = 1.00)

A-1507384 3

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253632	H09A	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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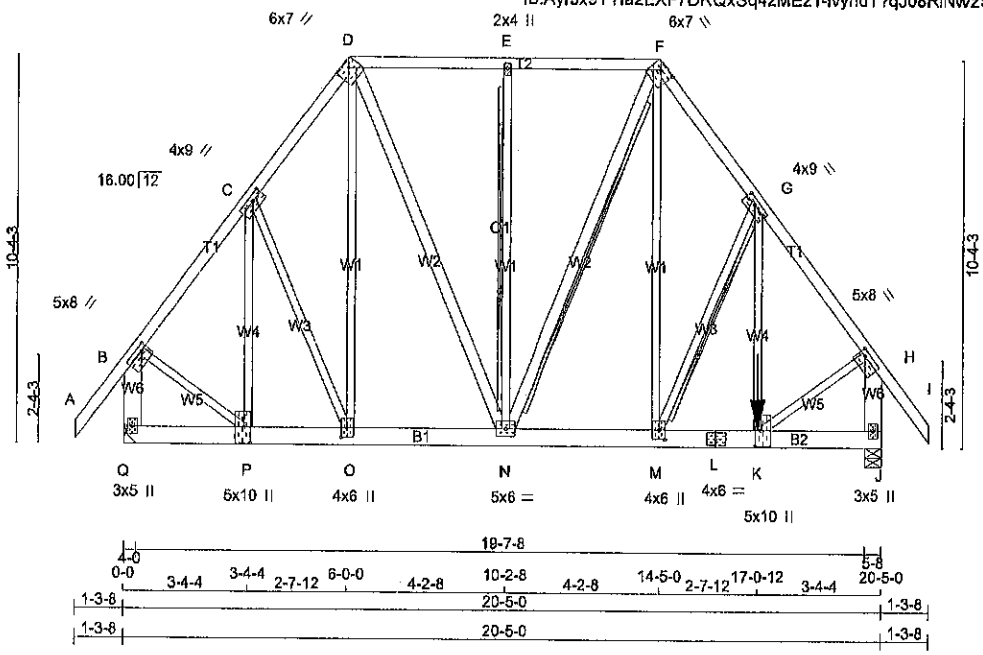

TOTAL WEIGHT = 165 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
Q - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
Q - L	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	1.50	2.00
C	TMWW-t	MT20	4.0	9.0	1.50	3.75
D	TTWW-h	MT20	6.0	7.0	1.75	4.00
E	TMW+w	MT20	2.0	4.0		
F	TTWW-h	MT20	6.0	7.0	1.75	4.00
G	TMWW-t	MT20	4.0	9.0	1.50	3.75
H	TMVW-t	MT20	5.0	8.0	1.50	2.00
J	BMV1+p	MT20	3.0	5.0		
K	BMWW+t	MT20	5.0	10.0	5.00	2.00
L	BS-t	MT20	4.0	6.0		
M	BMWW+t	MT20	4.0	6.0	3.00	1.75
N	BMWWW-t	MT20	5.0	6.0		
O	BMWW+t	MT20	4.0	6.0	3.00	1.75
P	BMWW+t	MT20	5.0	10.0	5.00	2.00
Q	BMV1+p	MT20	3.0	5.0		

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 2164.4 lbs FACTORED DOWN AT 17-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	1390	0	1390	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
J	3134	0	3134	0	0	5-8	4-11

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1010	508 / 0	0 / 0	0 / 0	0 / 0	502 / 0	0 / 0
J	2363	722 / 0	0 / 0	0 / 0	0 / 0	1641 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.91FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

2x4 DRY SPF No.2 T-BRACE REQUIRED AT E-N, F-N, G-M
FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-70.5	-70.5	0.13 (1)	10.00	P- C	-294 / 0	0.22 (1)
B- C	-1009 / 0	-70.5	-70.5	0.16 (1)	5.99	C- O	0 / 65	0.02 (4)
C- D	-1069 / 0	-70.5	-70.5	0.16 (1)	5.86	O- D	0 / 42	0.01 (1)
D- E	-875 / 0	-78.0	-78.0	0.30 (1)	6.09	D- N	0 / 650	0.13 (4)
E- F	-875 / 0	-78.0	-78.0	0.30 (1)	6.09	N- E	-384 / 0	0.33 (1)
F- G	-1690 / 0	-70.5	-70.5	0.32 (1)	4.80	N- F	-316 / 0	0.28 (4)
G- H	-2262 / 0	-198.1	-198.1	0.52 (1)	3.91	M- F	0 / 1199	0.44 (4)
H- I	0 / 40	-70.5	-70.5	0.13 (1)	10.00	M- G	-1160 / 0	0.50 (4)
Q- B	-1361 / 0	0.0	0.0	0.13 (1)	7.81	K- G	0 / 1521	0.58 (4)
J- H	-2997 / 0	0.0	0.0	0.30 (1)	5.97	B- P	0 / 706	0.20 (1)
						K- H	0 / 1580	0.48 (4)
Q- P	0 / 0	-17.5	-17.5	0.03 (4)	10.00			
P- O	0 / 616	-17.5	-17.5	0.10 (1)	10.00			
O- N	0 / 622	-17.5	-17.5	0.14 (4)	10.00			
N- M	0 / 931	-17.5	-17.5	0.20 (4)	10.00			
M- L	0 / 1379	-17.5	-17.5	0.44 (4)	10.00			
L- K	0 / 1379	-17.5	-17.5	0.44 (4)	10.00			
K- J	0 / 0	-49.2	-49.2	0.23 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
K	17-0-12	-1933	-1933	---	FRONT	VERT	DEAD

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND 6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 0-0
END SETBACK = 0-3-0
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 3-4-4 OF SPAN MEASURED FROM THE RIGHT.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.52 (G-H:1) , BC=0.44 (K-M:4) , WB=0.58 (G-K:4) , SSI=0.22 (G-H:1)

DOL LUMBER=0.88 NAIL=0.88 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

A-1013843

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253632	H09A	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (J) (INPUT = 0.90)
JSI METAL= 0.56 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07-17-2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

A-1073842(2)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253632	H09	7	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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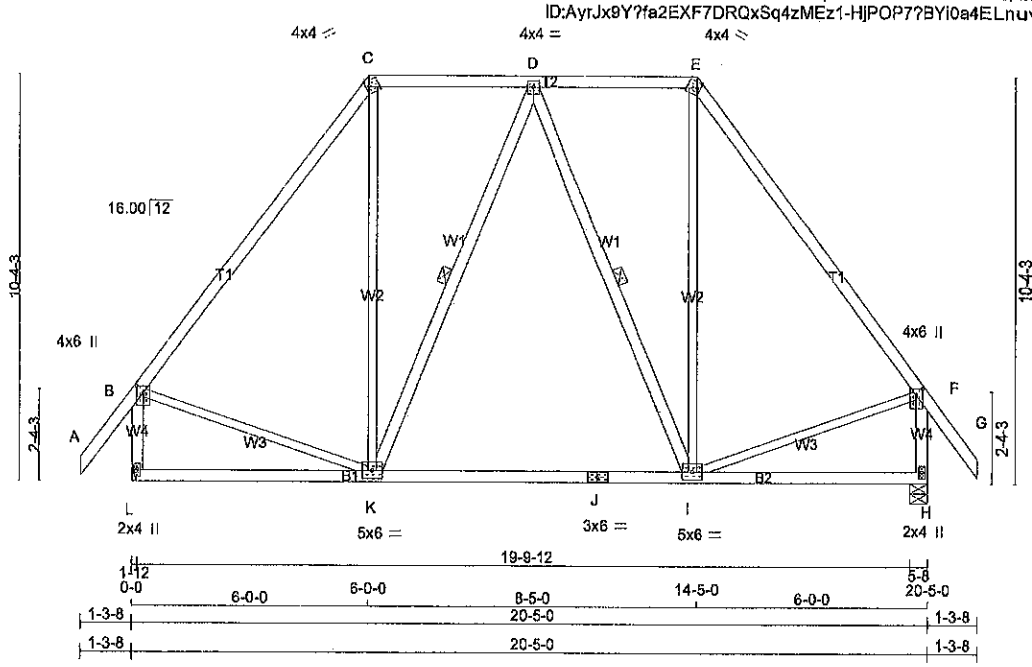

TOTAL WEIGHT = 7 X 116 = 810 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	1650F 1.5E	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	1650F 1.5E	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
K - D	2x4	DRY	No.2	SPF
D - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TTW-m	MT20	4.0	4.0	1.75	1.00
D	TMWW-t	MT20	4.0	4.0		
E	TTW-m	MT20	4.0	4.0	1.75	1.00
F	TMVW+p	MT20	4.0	6.0	2.00	2.00
H	BMV1+p	MT20	2.0	4.0		
I	BMWWW-t	MT20	5.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	5.0	6.0		
L	BMV1+p	MT20	2.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1029	0	1029	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
H	1029	0	1029	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	725	488 / 0	0 / 0	0 / 0	0 / 0	238 / 0	0 / 0
H	726	488 / 0	0 / 0	0 / 0	0 / 0	238 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF D-K, D-I.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 40	-70.5	-70.5	0.08 (1)	10.00	
B- C	-684 / 0	-70.5	-70.5	0.29 (1)	6.25	
C- D	-410 / 0	-78.0	-78.0	0.17 (1)	6.25	
D- E	-410 / 0	-78.0	-78.0	0.17 (1)	6.25	
E- F	-684 / 0	-70.5	-70.5	0.29 (1)	6.25	
F- G	0 / 40	-70.5	-70.5	0.08 (1)	10.00	
L- B	-990 / 0	0.0	0.0	0.13 (1)	7.81	
H- F	-990 / 0	0.0	0.0	0.13 (1)	7.81	
L- K	0 / 0	-17.5	-17.5	0.21 (4)	10.00	
K- J	0 / 492	-17.5	-17.5	0.26 (4)	10.00	
J- I	0 / 492	-17.5	-17.5	0.26 (4)	10.00	
I- H	0 / 0	-17.5	-17.5	0.21 (4)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	0 / 207	0.05 (4)
K- D	-214 / 0	0.14 (1)
D- I	-214 / 0	0.14 (1)
I- E	0 / 207	0.05 (4)
B- K	0 / 429	0.10 (1)
I- F	0 / 429	0.10 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	21.0	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.29 (B-C:1) , BC=0.26 (I-K:4) , WB=0.14 (D-I:1) , SSI=0.16 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (K) (INPUT = 0.80)
JSI METAL= 0.20 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07.17.2015
R. TURENNE
100157840
PROVINCE OF ONTARIO

A-N073841

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253632	H08A	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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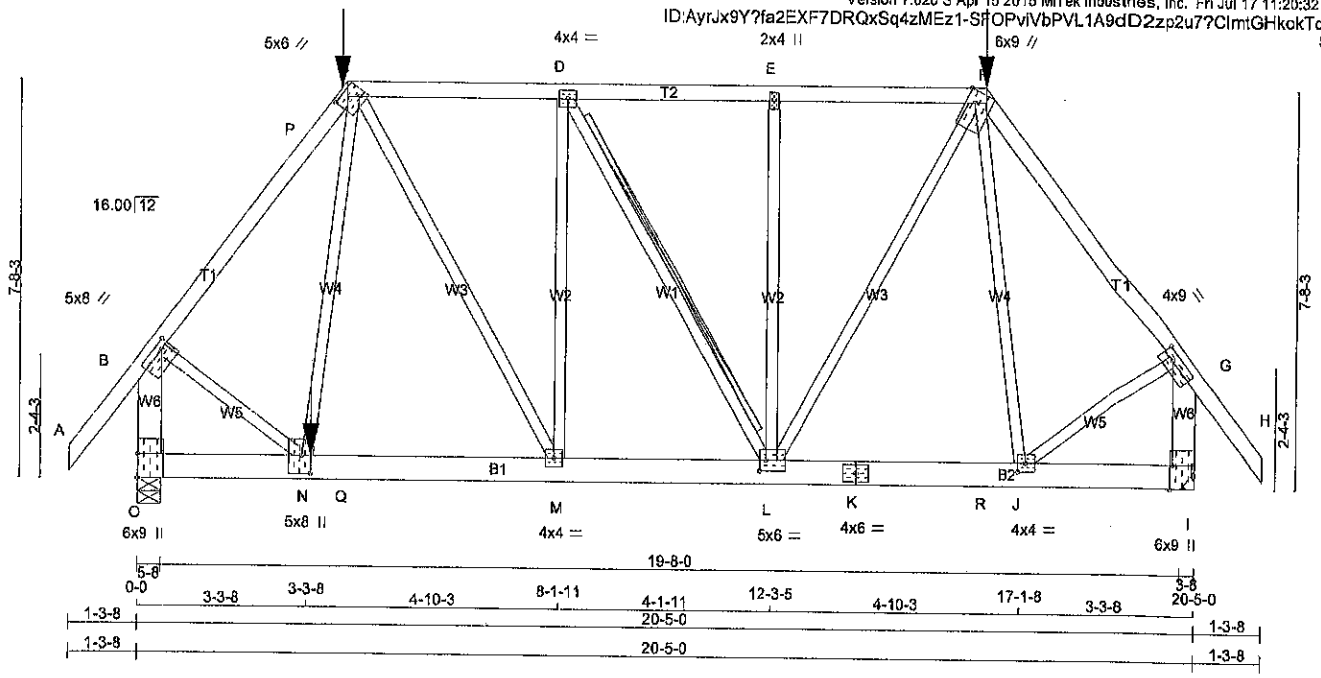


TOTAL WEIGHT = 127 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
O - B	2x6	DRY	No.2	SPF
I - G	2x6	DRY	No.2	SPF
O - K	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	1.50	2.00
C	TTWW-h	MT20	5.0	6.0	3.50	4.50
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	6.0	9.0	Edge	2.00
G	TMVW-t	MT20	4.0	9.0	1.75	3.00
I	BMV1+p	MT20	6.0	9.0	Edge	0.50
J	BMWW-t	MT20	4.0	4.0	2.00	1.75
K	BS-t	MT20	4.0	6.0		
L	BMWWW-t	MT20	5.0	6.0	2.50	1.50
M	BMWW-t	MT20	4.0	4.0		
N	BMWW+t	MT20	6.0	8.0	4.25	2.00
O	BMV1+t	MT20	6.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES
1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
O	3754	0	3754	0	0	5-8	5-8
I	1817	0	1817	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-15	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	2643	1802 / 0	0 / 0	0 / 0	0 / 0	842 / 0	0 / 0
I	1278	876 / 0	0 / 0	0 / 0	0 / 0	402 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.14FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

2x4 DRY SPF No.2 T-BRACE REQUIRED AT D-L
FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-70.5	-70.5	0.11 (1)	10.00	N- C	0 / 1357	0.34 (1)
B- P	-2772 / 0	-198.1	-198.1	0.93 (1)	3.14	C- M	0 / 264	0.08 (1)
P- C	-2772 / 0	-70.5	-70.5	0.93 (1)	3.14	M- D	-45 / 79	0.05 (1)
C- D	-1626 / 0	-105.7	-105.7	0.32 (1)	4.80	D- L	-480 / 0	0.26 (1)
D- E	-1389 / 0	-105.7	-105.7	0.31 (1)	5.12	L- E	-459 / 0	0.48 (1)
E- F	-1389 / 0	-105.7	-105.7	0.29 (1)	5.15	L- F	0 / 1105	0.27 (1)
F- G	-1356 / 0	-70.5	-70.5	0.28 (1)	5.26	F- J	-326 / 0	0.35 (1)
G- H	0 / 40	-70.5	-70.5	0.11 (1)	10.00	B- N	0 / 1889	0.47 (1)
O- B	-3633 / 0	0.0	0.0	0.32 (1)	5.55	J- G	0 / 933	0.23 (1)
I- G	-1793 / 0	0.0	0.0	0.16 (1)	7.41			
O- N	0 / 0	-56.0	-56.0	0.09 (1)	10.00			
N- Q	0 / 1500	-26.3	-26.3	0.27 (1)	10.00			
Q- M	0 / 1500	-26.3	-26.3	0.27 (1)	10.00			
M- L	0 / 1626	-26.3	-26.3	0.27 (1)	10.00			
L- K	0 / 842	-26.3	-26.3	0.16 (1)	10.00			
K- R	0 / 842	-26.3	-26.3	0.16 (1)	10.00			
R- J	0 / 842	-26.3	-26.3	0.16 (1)	10.00			
J- I	0 / 0	-26.3	-26.3	0.06 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	4-0-0	-117	-117	---	FRONT	VERT	TOTAL
F	16-5-0	-117	-117	---	FRONT	VERT	TOTAL
N	3-4-4	-2194	-2194	---	FRONT	VERT	TOTAL
O	0-0	-9	-9	---	FRONT	VERT	DEAD

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 0-0
END SETBACK = 9-3-0
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 3-4-4 OF SPAN MEASURED FROM THE LEFT.

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 4-0-0
END SETBACK = 4-0-0
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 65 % OF GSL.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.93 (B-C:1) , BC=0.27 (L-M:1) , WB=0.48 (E-L:1) , SSI=0.22 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

A-1013840

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253632	H08A	1	1	TRUSS DESC.	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 117.4 lbs FACTORED DOWN AT 16-5-0, AND 117.4 lbs FACTORED DOWN AT 4-0-0 ON TOP CHORD, AND 2194.3 lbs FACTORED DOWN AT 3-4-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

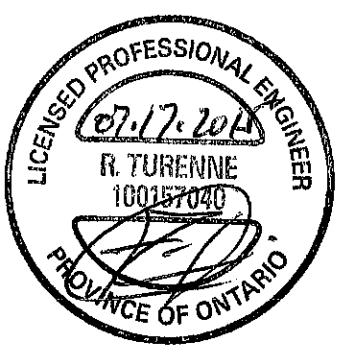
NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2294	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.64 (C) (INPUT = 1.00)



A-1507384O(2)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253632	H08	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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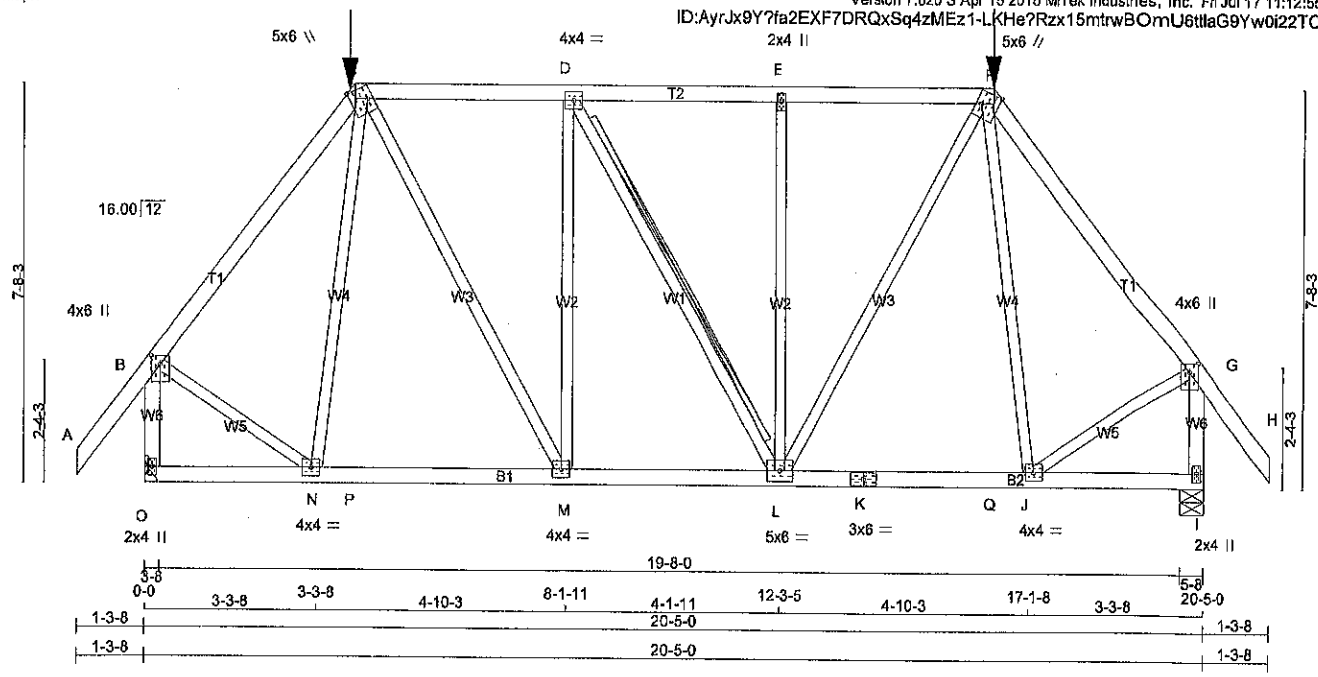

TOTAL WEIGHT = 112 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TTWW+m	MT20	5.0	6.0	Edge	3.75
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	Edge	3.75
G	TMVW+p	MT20	4.0	6.0	2.00	2.00
I	BMV1+p	MT20	2.0	4.0		
J, M, N						
J	BMWW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	5.0	6.0		
O	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 117.4 lbs FACTORED DOWN AT 16-5-0, AND 117.4 lbs FACTORED DOWN AT 4-0-0 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REORD BRG IN-SX
O	1422	0	1422	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-9	
I	1422	0	1422	0	0	5-8	1-9

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1000	688 / 0	0 / 0	0 / 0	0 / 0	313 / 0	0 / 0
I	1000	688 / 0	0 / 0	0 / 0	0 / 0	313 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.94FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

2x4 DRY SPF No.2 T-BRACE REQUIRED AT D-I.
FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 40	-70.5	-70.5	0.11 (1)	10.00	N-C	-236 / 20	0.27 (1)
B-C	-995 / 0	-70.5	-70.5	0.25 (1)	5.94	C-M	0 / 673	0.17 (1)
C-D	-946 / 0	-105.7	-105.7	0.28 (1)	5.98	M-D	-467 / 0	0.51 (1)
D-E	-945 / 0	-105.7	-105.7	0.28 (1)	5.97	D-L	-4 / 0	0.00 (1)
E-F	-944 / 0	-105.7	-105.7	0.27 (1)	5.99	L-E	-466 / 0	0.51 (1)
F-G	-995 / 0	-70.5	-70.5	0.25 (1)	5.94	L-F	0 / 668	0.17 (1)
G-H	0 / 40	-70.5	-70.5	0.11 (1)	10.00	F-J	-232 / 20	0.26 (1)
O-B	-1394 / 0	0.0	0.0	0.19 (1)	6.86	B-N	0 / 691	0.17 (1)
I-G	-1394 / 0	0.0	0.0	0.19 (1)	6.86	J-G	0 / 692	0.17 (1)
O-N	0 / 0	-26.3	-26.3	0.12 (4)	10.00			
N-P	0 / 816	-26.3	-26.3	0.18 (1)	10.00			
P-M	0 / 816	-26.3	-26.3	0.18 (1)	10.00			
M-L	0 / 947	-26.3	-26.3	0.23 (1)	10.00			
L-K	0 / 616	-26.3	-26.3	0.18 (1)	10.00			
K-Q	0 / 616	-26.3	-26.3	0.18 (1)	10.00			
Q-J	0 / 616	-26.3	-26.3	0.18 (1)	10.00			
J-I	0 / 0	-26.3	-26.3	0.12 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	4-0-0	-117	-117	---	FRONT	VERT	TOTAL
F	16-5-0	-117	-117	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 4-0-0
END SETBACK = 4-0-0
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.28 (C-D:1) , BC=0.23 (L-M:1) , WB=0.51 (D-M:1) , SSI=0.22 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.60

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2204	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (G) (INPUT = 0.90)
JSI METAL= 0.26 (I) (INPUT = 1.00)

A-H073839

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253632	H04A	1	2	TRUSS DESC.	

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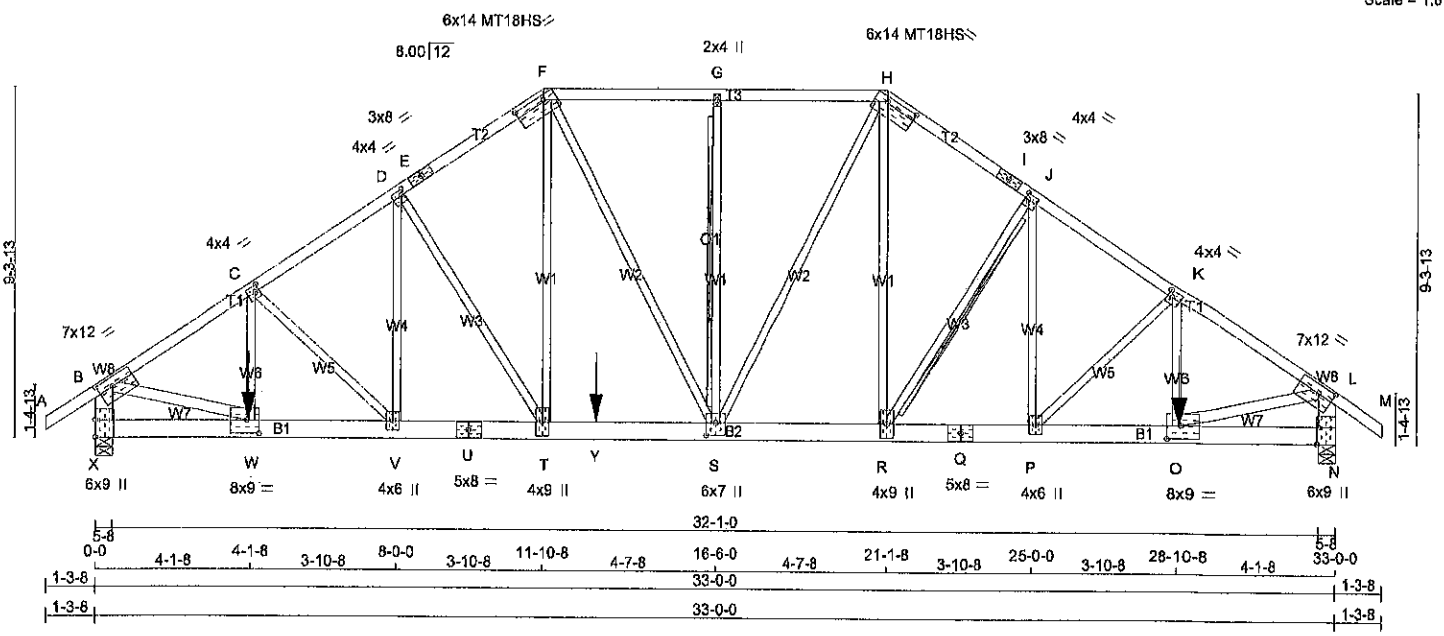

TOTAL WEIGHT = 2 X 189 = 379 lb

(M)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - I	2x4	DRY	No.2	SPF
I - M	2x4	DRY	No.2	SPF
X - B	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
X - U	2x6	DRY	1650F 1.5E	SPF
U - Q	2x6	DRY	1650F 1.5E	SPF
Q - N	2x6	DRY	1650F 1.5E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
B - W	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- E	1	12	TOP
E- F	1	12	TOP
F- H	1	12	TOP
H- I	1	12	TOP
I- M	1	12	TOP
X- B	2	12	TOP
N- L	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
X- U	2	12	SIDE(217.9)
U- Q	2	9	SIDE(257.3)
Q- N	2	9	SIDE(257.3)
WEBS : (0.122"X3") SPIRAL NAILS			
C- W	1	4	SIDE(416.1)
2x3	1	5	SIDE(344.6)
2x3	1	6	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
X	10129	0	10129	0	0	5-8	5-8
N	9897	0	9897	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
X	7137	4628 / 0	0 / 0	0 / 0	0 / 0	2309 / 0	0 / 0
N	6974	4718 / 0	0 / 0	0 / 0	0 / 0	2257 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.14FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

2x4 DRY SPF No.2 T-BRACE REQUIRED AT G-S, J-R
FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 27	-70.5	-70.5	0.05 (1)	10.00	W- C	0 / 74	0.01 (4)
B- C	-12969 / 0	-70.5	-70.5	0.84 (1)	2.14	C- V	-285 / 0	0.07 (1)
C- D	-12721 / 0	-70.5	-70.5	0.72 (1)	2.25	V- D	0 / 1868	0.23 (1)
D- E	-11683 / 0	-70.5	-70.5	0.64 (1)	2.46	D- T	-1834 / 0	1.00 (1)
E- F	-11683 / 0	-70.5	-70.5	0.64 (1)	2.46	T- F	0 / 4753	0.59 (1)
F- G	-10389 / 0	-78.0	-78.0	0.56 (1)	2.68	F- S	0 / 1567	0.19 (1)
G- H	-10389 / 0	-78.0	-78.0	0.56 (1)	2.68	S- G	-359 / 0	0.15 (1)
H- I	-11236 / 0	-70.5	-70.5	0.61 (1)	2.53	S- H	0 / 2214	0.27 (1)
I- J	-11236 / 0	-70.5	-70.5	0.61 (1)	2.53	R- H	0 / 3986	0.49 (1)
J- K	-12592 / 0	-70.5	-70.5	0.71 (1)	2.28	R- J	-2175 / 0	0.59 (1)
K- L	-12675 / 0	-70.5	-70.5	0.81 (1)	2.20	P- J	0 / 2278	0.28 (1)
L- M	0 / 27	-70.5	-70.5	0.05 (1)	10.00	P- K	-93 / 0	0.02 (1)
X- B	-10040 / 0	0.0	0.0	0.36 (1)	4.76	O- K	-169 / 2	0.02 (1)
N- L	-9817 / 0	0.0	0.0	0.35 (1)	4.81	B- W	0 / 11105	0.98 (1)
						O- L	0 / 10852	0.96 (1)
X- W	0 / 0	-17.5	-62.2	0.04 (1)	10.00			
W- V	0 / 10794	-453.2	-453.2	0.63 (1)	10.00			
V- U	0 / 10689	-453.2	-453.2	0.72 (1)	10.00			
U- T	0 / 10689	-453.2	-453.2	0.72 (1)	10.00			
T- Y	0 / 9669	-453.2	-453.2	0.73 (1)	10.00			
Y- S	0 / 9669	-532.0	-532.0	0.73 (1)	10.00			
S- R	0 / 9371	-532.0	-532.0	0.72 (1)	10.00			
R- Q	0 / 10482	-532.0	-532.0	0.60 (1)	10.00			
Q- P	0 / 10482	-532.0	-532.0	0.60 (1)	10.00			
P- O	0 / 10549	-532.0	-532.0	0.63 (1)	10.00			
O- N	0 / 0	-62.2	-17.5	0.03 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
O	28-11-4	-1419	-1419	---	FRONT	VERT	TOTAL
W	4-0-12	-1814	-1814	---	FRONT	VERT	TOTAL
X	0-0	---	---	---	FRONT	VERT	TOTAL
Y	13-3-12	-1434	-1434	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 2-0-0
END DISTANCE = 4-0-12
END SPAN CARRIED = 4-0-0
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

GIRDER TYPE: CStdGirder
START DISTANCE = 28-11-4
START SPAN CARRIED = 4-0-0
END DISTANCE = 33-0-0
END SPAN CARRIED = 2-0-0
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.23")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 948 (0.42")

CSI: TC=0.84 (B-C:1) , BC=0.73 (S-T:1) , WB=1.00 (D-T:1) , SSI=0.69 (S-T:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

CONTINUED ON PAGE 2

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	7.0	12.0	2.75	4.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TMWW-t	MT20	4.0	4.0	1.75	1.00
E	TS-t	MT20	3.0	8.0		
F	TTWW-h	MT18HS	6.0	14.0	1.50	10.00
G	TMW+w	MT20	2.0	4.0		
H	TTWW-h	MT18HS	6.0	14.0	1.50	10.00
I	TS-t	MT20	3.0	8.0		
J	TMWW-t	MT20	4.0	4.0	1.75	1.00
K	TMWW-t	MT20	4.0	4.0	2.00	1.50
L	TMVW-t	MT20	7.0	12.0	2.75	4.75
N	BMV1+t	MT20	6.0	9.0	Edge	0.50
O	BMWW-t	MT20	6.0	9.0	4.25	4.00
P	BMWW+t	MT20	4.0	6.0		
Q	BS-t	MT20	5.0	8.0		
R	BMWW+t	MT20	4.0	9.0		
S	BMWWW+t	MT20	6.0	7.0	4.00	3.00
T	BMWW+t	MT20	4.0	9.0		
U	BS-t	MT20	5.0	8.0		
V	BMWW+t	MT20	4.0	6.0		
W	BMWW-t	MT20	6.0	9.0	4.25	4.00
X	BMV1+t	MT20	6.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1813.9 lbs FACTORED DOWN AT 4-0-12, AND 1433.5 lbs FACTORED DOWN AT 13-3-12, AND 1419.3 lbs FACTORED DOWN AT 28-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656
MT18HS	580	354	2729	1405	4191	2010

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (D) (INPUT = 0.90)
JSI METAL= 0.99 (B) (INPUT = 1.00)

A-1073838(2)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253630	J01C	6	1	TRUSS DESC.	

Alpa Roof Truss, Maple

ID:hdKFA8XS_E1ZIH0GL9ahDNzQR2b-_67QqeREXRVoeHfIMVZBgkCLrFu8BLNdEI35J8yxD5A

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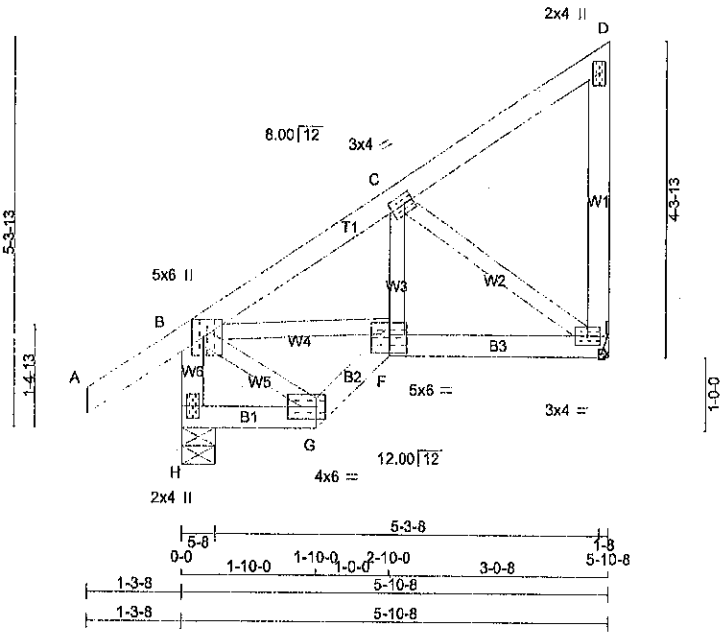

TOTAL WEIGHT = 6 X 30 = 182 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
H - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVWW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	3.0	4.0	1.50	1.50
D	TMV+p	MT20	2.0	4.0		
E	BMVW1-t	MT20	3.0	4.0		
F	BBWW-I	MT20	5.0	6.0	3.00	3.00
G	BBW-I	MT20	4.0	6.0	2.00	4.50
H	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	355	0	355	0	0	5-8	1-8
E	258	0	258	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	248	181 / 0	0 / 0	0 / 0	0 / 0	67 / 0	0 / 0
E	182	123 / 0	0 / 0	0 / 0	0 / 0	59 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- B	-339 / 0	0.0	0.0	0.03 (1)		7.81	B- G	0 / 20	0.01 (4)
A- B	0 / 27	-70.5	-70.5	0.09 (1)		10.00	B- F	0 / 209	0.05 (1)
B- C	-257 / 0	-70.5	-70.5	0.10 (1)		6.25	F- C	0 / 57	0.02 (4)
C- D	-13 / 0	-70.5	-70.5	0.10 (1)		6.25	C- E	-271 / 0	0.06 (1)
E- D	-79 / 0	0.0	0.0	0.02 (1)		7.81			
H- G	0 / 0	-17.5	-17.5	0.02 (4)		10.00			
G- F	0 / 25	-17.5	-17.5	0.01 (4)		10.00			
F- E	0 / 223	-17.5	-17.5	0.06 (1)		10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.10 (B-C:1) , BC=0.06 (E-F:1) , WB=0.06 (C-E:1) , SSI=0.08 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.52 (B) (INPUT = 0.90)
JSI METAL= 0.08 (E) (INPUT = 1.00)

A-1507383

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253630	J01B	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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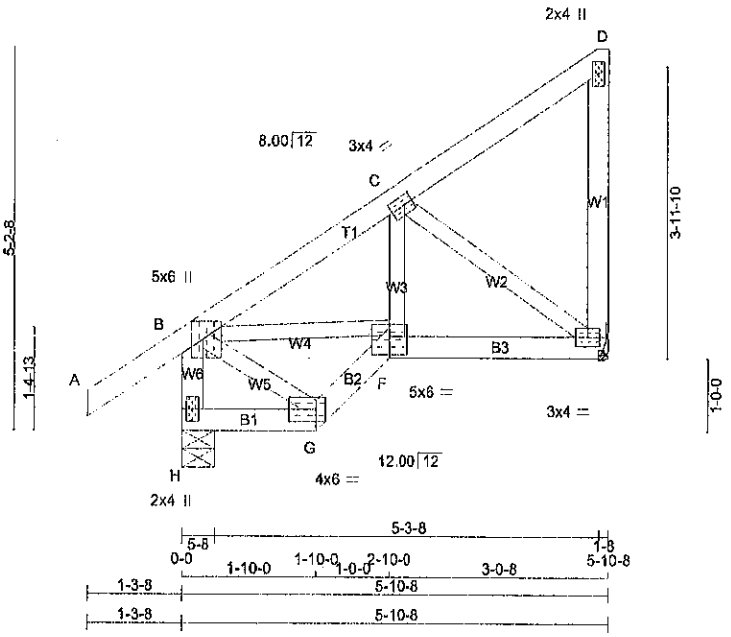

TOTAL WEIGHT = 30 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVWW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	3.0	4.0	1.50	1.50
D	TMV+p	MT20	2.0	4.0		
E	BMVW1-t	MT20	3.0	4.0		
F	BBWW-I	MT20	5.0	6.0	3.00	3.00
G	BBW-I	MT20	4.0	6.0	2.00	4.50
H	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
E	258	0	258	0	0	IN-SX	IN-SX
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	
H	355	0	355	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	182	123 / 0	0 / 0	0 / 0	0 / 0	59 / 0	0 / 0
H	248	181 / 0	0 / 0	0 / 0	0 / 0	67 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 27	-70.5	-70.5	0.09 (1)	10.00	B- F	0 / 209	0.05 (1)	
B- C	-257 / 0	-70.5	-70.5	0.10 (1)	6.26	F- C	0 / 57	0.02 (4)	
C- D	-13 / 0	-70.5	-70.5	0.10 (1)	6.25	C- E	-271 / 0	0.06 (1)	
E- D	-79 / 0	0.0	0.0	0.02 (1)	7.81	B- G	0 / 20	0.01 (4)	
H- B	-339 / 0	0.0	0.0	0.03 (1)	7.81				
H- G	0 / 0	-17.5	-17.5	0.02 (4)	10.00				
G- F	0 / 25	-17.5	-17.5	0.01 (4)	10.00				
F- E	0 / 223	-17.5	-17.5	0.06 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.10 (B-C:1) , BC=0.06 (E-F:1) , WB=0.06 (C-E:1) , SSI=0.08 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.52 (B) (INPUT = 0.90)
JSI METAL= 0.08 (E) (INPUT = 1.00)

A-1073836

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253630	J01A	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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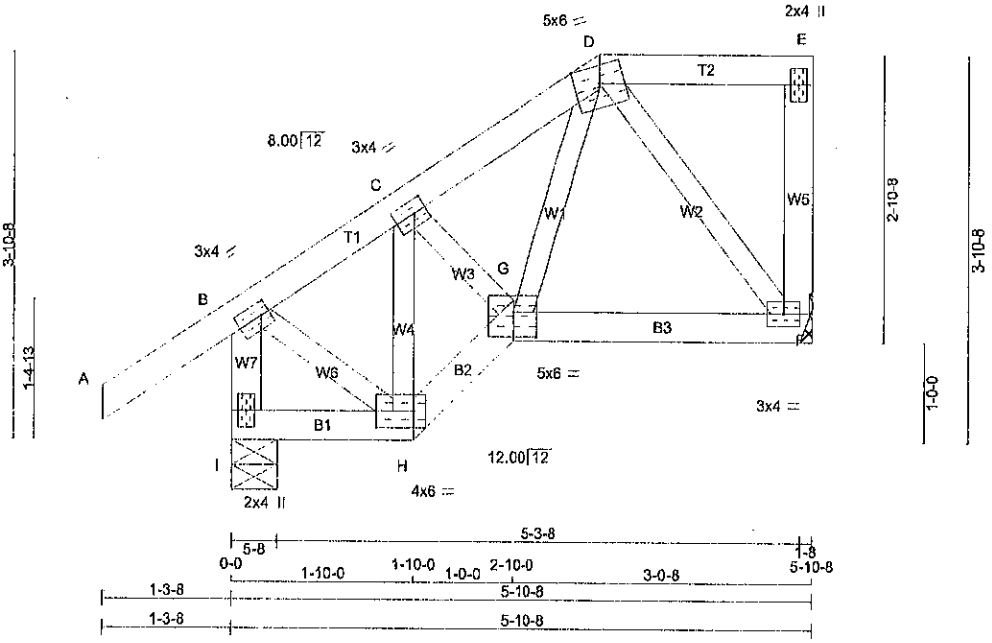

TOTAL WEIGHT = 29 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	3.0	4.0	1.50	1.00
C	TMWW-t	MT20	3.0	4.0	1.50	1.50
D	TTWW-m	MT20	5.0	6.0	Edge	
E	TMV+p	MT20	2.0	4.0		
F	BMVW1-t	MT20	3.0	4.0		
G	BBWW-t	MT20	5.0	6.0	3.00	3.00
H	BBWW-t	MT20	4.0	6.0	2.00	4.50
I	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	247	0	247	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
I	366	0	366	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	174	117 / 0	0 / 0	0 / 0	0 / 0	58 / 0	0 / 0
I	256	188 / 0	0 / 0	0 / 0	0 / 0	68 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 27	-70.5	-70.5	0.09 (1)	10.00	H- C	-170 / 0	0.03 (1)
B- C	-173 / 0	-70.5	-70.5	0.09 (1)	6.25	C- G	0 / 61	0.01 (1)
C- D	-197 / 0	-70.5	-70.5	0.04 (1)	6.25	G- D	0 / 119	0.03 (1)
D- E	0 / 0	-70.5	-70.5	0.06 (1)	10.00	D- F	-190 / 0	0.04 (1)
F- E	-76 / 0	0.0	0.0	0.01 (1)	7.81	B- H	0 / 148	0.03 (1)
I- B	-350 / 0	0.0	0.0	0.04 (1)	7.81			
I- H	0 / 0	-17.5	-17.5	0.02 (4)	10.00			
H- G	0 / 171	-17.5	-17.5	0.03 (1)	10.00			
G- F	0 / 123	-17.5	-17.5	0.06 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.09 (A-B:1) , BC=0.06 (F-G:4) , WB=0.04 (D-F:1) , SSI=0.06 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.48 (B) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

A-1207385

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253630	H04T	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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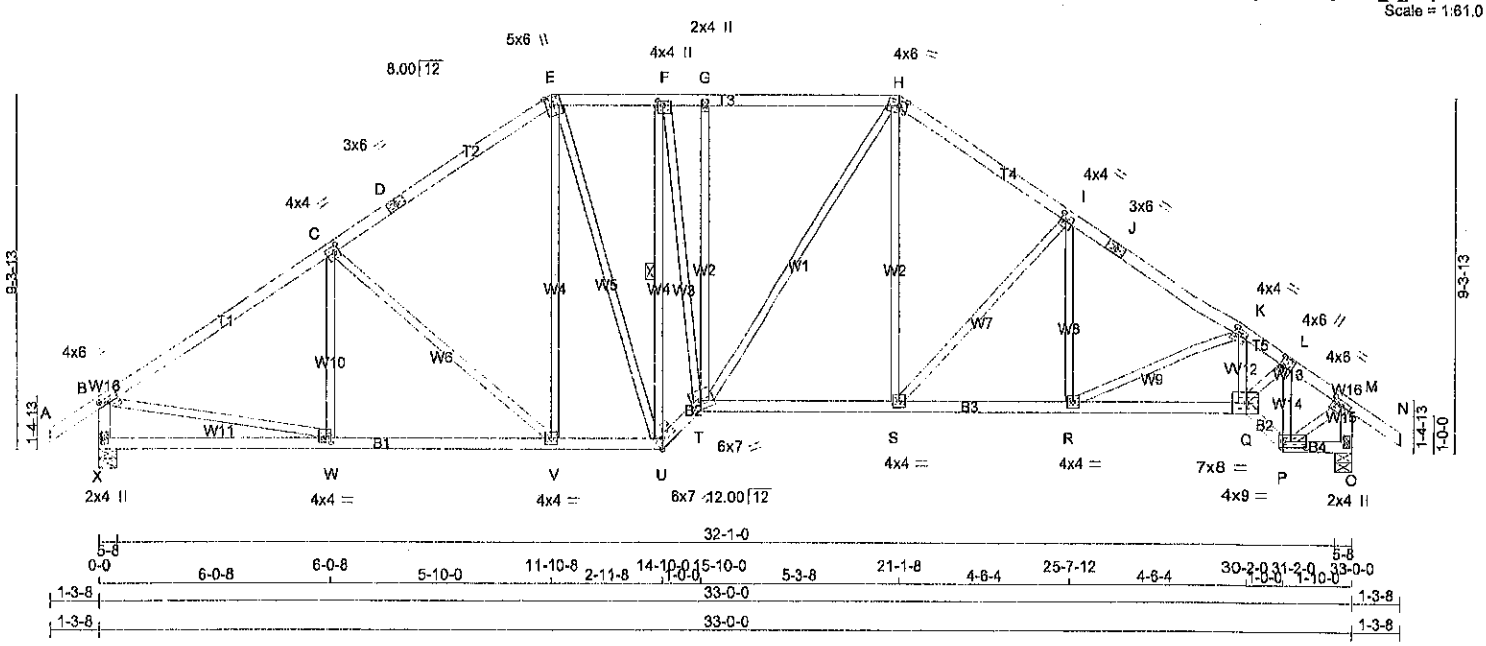

TOTAL WEIGHT = 2 X 170 = 339 lb
[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
J - N	2x4	DRY	No.2	SPF
X - B	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
X - U	2x4	DRY	No.2	SPF
U - T	2x4	DRY	No.2	SPF
T - Q	2x4	DRY	No.2	SPF
Q - P	2x4	DRY	No.2	SPF
P - O	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	1.75	3.00
C, I, K						
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTWW+m	MT20	5.0	6.0	Edge	1.75
F	TMWW+t	MT20	4.0	4.0	1.50	1.50
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	4.0	6.0	1.75	2.50
J	TS-t	MT20	3.0	6.0		
L	TMWW+t	MT20	4.0	6.0	3.00	1.25
M	TMVW-t	MT20	4.0	6.0	1.50	3.00
O	BMV1+p	MT20	2.0	4.0		
P	BBWW-I	MT20	4.0	9.0	2.00	7.50
Q	BBWW-I	MT20	7.0	8.0		
R, S, V						
R	BMWW-t	MT20	4.0	4.0		
T	BBWWW-m	MT20	6.0	7.0	2.75	3.00
U	BBWW-h	MT20	6.0	7.0	Edge	2.50
W	BMWW-t	MT20	4.0	4.0	1.50	1.50
X	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
X	1546	0	1546	0	0	5-8	1-11
O	1550	0	1550	0	0	5-8	1-11

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
X	1087	749 / 0	0 / 0	0 / 0	0 / 0	338 / 0	0 / 0
O	1090	751 / 0	0 / 0	0 / 0	0 / 0	338 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.47FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF F-U.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX UNBRAC LENGTH
A-B	0 / 27	-70.5	-70.5	0.09 (1)	10.00	
B-C	-1739 / 0	-70.5	-70.5	0.40 (1)	4.67	
C-D	-1499 / 0	-70.5	-70.5	0.37 (1)	4.97	
D-E	-1499 / 0	-70.5	-70.5	0.37 (1)	4.97	
E-F	-1322 / 0	-70.5	-70.5	0.11 (1)	5.53	
F-G	-1505 / 0	-70.5	-70.5	0.15 (1)	5.22	
G-H	-1508 / 0	-70.5	-70.5	0.27 (1)	5.07	
H-I	-1670 / 0	-70.5	-70.5	0.20 (1)	4.96	
I-J	-2018 / 0	-70.5	-70.5	0.22 (1)	4.59	
J-K	-2018 / 0	-70.5	-70.5	0.22 (1)	4.59	
K-L	-2289 / 0	-70.5	-70.5	0.11 (1)	4.47	
L-M	-1306 / 0	-70.5	-70.5	0.09 (1)	5.58	
M-N	0 / 27	-70.5	-70.5	0.09 (1)	10.00	
X-B	-1502 / 0	0.0	0.0	0.15 (1)	6.70	
O-M	-1534 / 0	0.0	0.0	0.16 (1)	6.64	
X-W	0 / 0	-17.5	-17.5	0.15 (4)	10.00	
W-V	0 / 1469	-17.5	-17.5	0.31 (1)	10.00	
V-U	0 / 1226	-17.5	-17.5	0.25 (1)	10.00	
U-T	0 / 1892	-17.5	-17.5	0.31 (1)	10.00	
T-S	0 / 1378	-17.5	-17.5	0.28 (1)	10.00	
S-R	0 / 1689	-17.5	-17.5	0.33 (1)	10.00	
R-Q	0 / 1928	-17.5	-17.5	0.36 (1)	10.00	
Q-P	0 / 1388	-17.5	-17.5	0.23 (1)	10.00	
P-O	0 / 0	-17.5	-17.5	0.02 (4)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
W-C	-145 / 52	0.06 (1)
C-V	-326 / 0	0.41 (1)
V-E	0 / 286	0.06 (1)
E-U	0 / 307	0.07 (1)
U-F	-1596 / 0	0.91 (1)
F-T	0 / 1475	0.33 (1)
T-G	-271 / 0	0.37 (1)
T-H	0 / 236	0.05 (1)
S-H	0 / 432	0.10 (1)
S-I	-465 / 0	0.40 (1)
R-I	0 / 177	0.04 (4)
R-K	-264 / 0	0.10 (1)
Q-K	0 / 49	0.02 (4)
Q-L	0 / 1400	0.32 (1)
P-L	-1563 / 0	0.25 (1)
B-W	0 / 1493	0.34 (1)
P-M	0 / 1216	0.27 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.23")

CSI: TC=0.40 (B-C:1) , BC=0.36 (Q-R:1) , WB=0.91 (F-U:1) , SSI=0.17 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (W) (INPUT = 0.90)
JSI METAL= 0.55 (W) (INPUT = 1.00)

A-A072834

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253630	H03T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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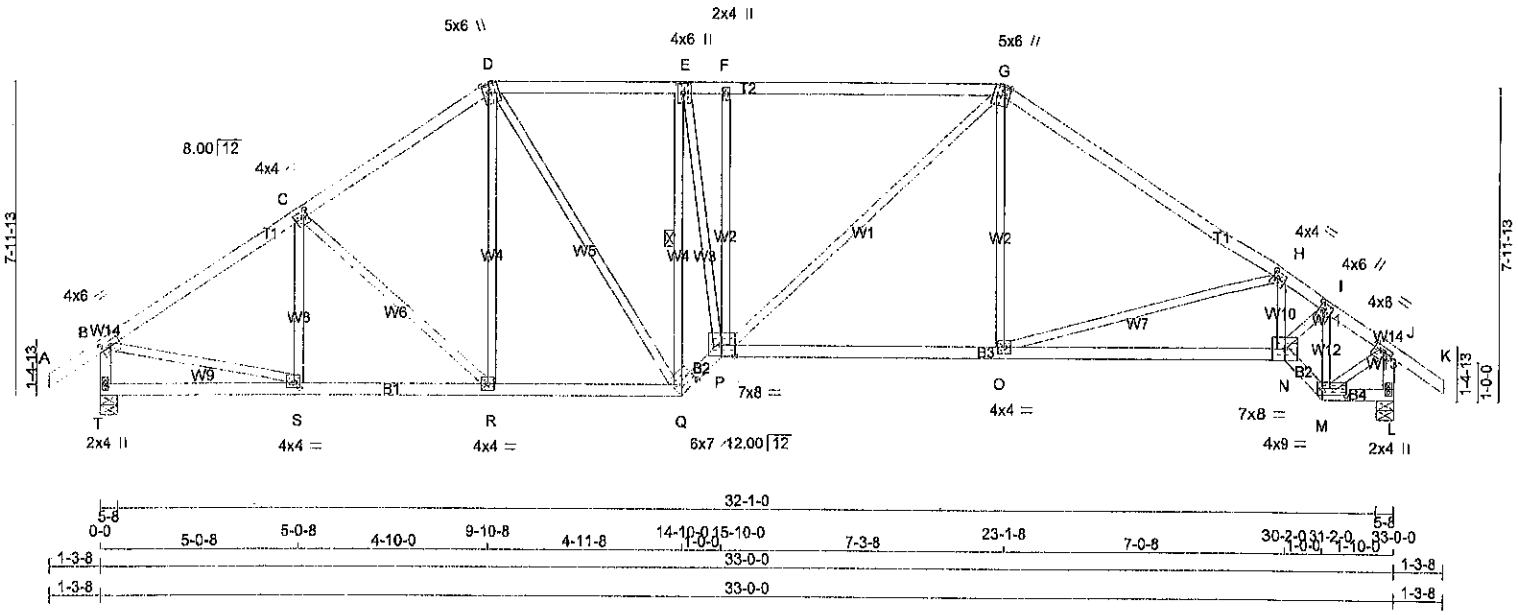

TOTAL WEIGHT = 164 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - K	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - Q	2x4	DRY	No.2	SPF
Q - P	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	1.75	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	Edge	1.75
E	TMWW+t	MT20	4.0	6.0	3.00	1.50
F	TMW+w	MT20	2.0	4.0		
G	TTWW+m	MT20	5.0	6.0	2.50	1.50
H	TMWW-t	MT20	4.0	4.0	2.00	1.50
I	TMWW+t	MT20	4.0	6.0	2.50	1.25
J	TMVW-t	MT20	4.0	6.0	1.50	3.00
L	BMV1+p	MT20	2.0	4.0		
M	BBWW-t	MT20	4.0	9.0	2.00	7.50
N	BBWW-t	MT20	7.0	8.0		
O	BMWW-t	MT20	4.0	4.0		
P	BBWWW-p	MT20	7.0	8.0	3.25	4.25
Q	BBWW-h	MT20	6.0	7.0	2.25	4.50
R	BMWW-t	MT20	4.0	4.0		
S	BMWW-t	MT20	4.0	4.0	1.50	1.50
T	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	1546	0	1546	0	0	5-8	1-11
L	1550	0	1550	0	0	5-8	1-11

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1087	749 / 0	0 / 0	0 / 0	0 / 0	338 / 0	0 / 0
L	1090	751 / 0	0 / 0	0 / 0	0 / 0	338 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.11FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF E-Q.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 27	-70.5	-70.5	0.09 (1)	10.00
B- C	-1715 / 0	-70.5	-70.5	0.26 (1)	4.85
C- D	-1598 / 0	-70.5	-70.5	0.25 (1)	4.99
D- E	-1585 / 0	-70.5	-70.5	0.24 (1)	5.04
E- F	-1835 / 0	-70.5	-70.5	0.37 (1)	4.54
F- G	-1842 / 0	-70.5	-70.5	0.51 (1)	4.40
G- H	-1843 / 0	-70.5	-70.5	0.52 (1)	4.44
H- I	-2272 / 0	-70.5	-70.5	0.41 (1)	4.11
I- J	-1294 / 0	-70.5	-70.5	0.13 (1)	5.55
J- K	0 / 27	-70.5	-70.5	0.09 (1)	10.00
T- B	-1507 / 0	0.0	0.0	0.16 (1)	6.68
L- J	-1534 / 0	0.0	0.0	0.16 (1)	6.64
T- S	0 / 0	-17.5	-17.5	0.09 (4)	10.00
S- R	0 / 1445	-17.5	-17.5	0.29 (1)	10.00
R- Q	0 / 1311	-17.5	-17.5	0.27 (1)	10.00
Q- P	0 / 2245	-17.5	-17.5	0.36 (1)	10.00
P- O	0 / 1514	-17.5	-17.5	0.39 (1)	10.00
O- N	0 / 2069	-17.5	-17.5	0.48 (1)	10.00
N- M	0 / 1355	-17.5	-17.5	0.22 (1)	10.00
M- L	0 / 0	-17.5	-17.5	0.02 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- C	-219 / 8	0.07 (1)
C- R	-184 / 0	0.15 (1)
R- D	0 / 212	0.05 (1)
D- Q	0 / 467	0.11 (1)
Q- E	-1928 / 0	0.77 (1)
E- P	0 / 1825	0.41 (1)
P- F	-457 / 0	0.38 (1)
P- G	0 / 444	0.10 (1)
O- G	0 / 302	0.07 (4)
O- H	-583 / 0	0.63 (1)
N- H	-176 / 41	0.03 (1)
N- I	0 / 1629	0.37 (1)
M- I	-1525 / 0	0.24 (1)
B- S	0 / 1478	0.33 (1)
M- J	0 / 1187	0.27 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.28")

CSI: TC=0.52 (G-H:1) , BC=0.48 (N-O:1) , WB=0.77 (E-Q:1) , SSI=0.27 (H-I:1) .

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.60

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (S) (INPUT = 0.90)
JSI METAL= 0.54 (S) (INPUT = 1.00)

A-1073833

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253630	H02T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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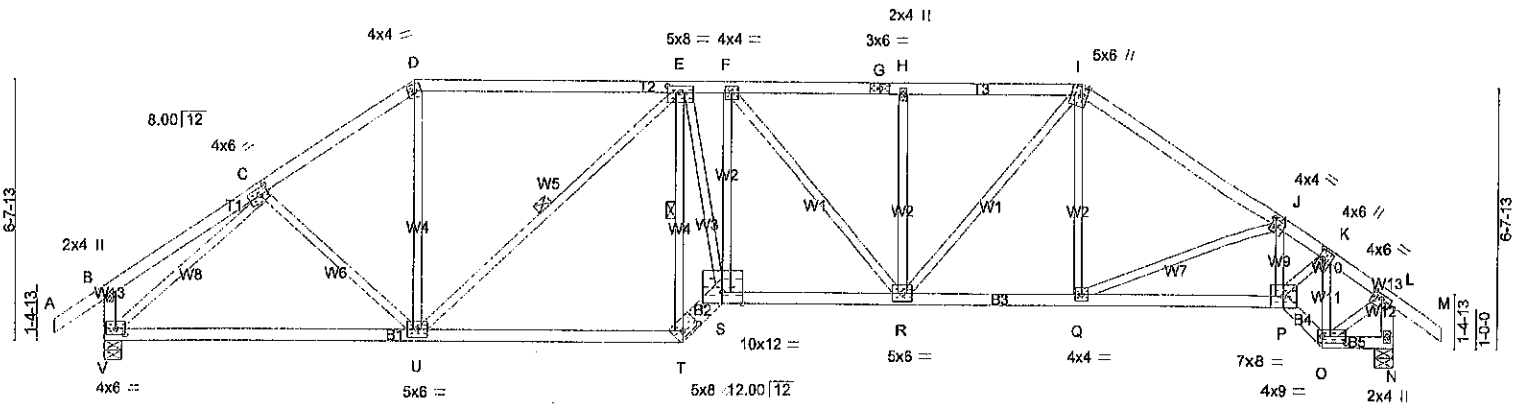

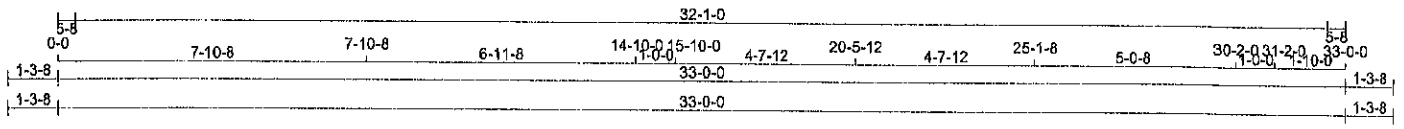

TOTAL WEIGHT = 150 lb
[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - M	2x4	DRY	No.2	SPF
V - B	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
V - T	2x4	DRY	No.2	SPF
T - S	2x6	DRY	No.2	SPF
S - P	2x4	DRY	No.2	SPF
P - O	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWW-t	MT20	4.0	6.0	2.00	2.50
D	TTW-m	MT20	4.0	4.0		
E	TMWWW-t	MT20	5.0	8.0	2.00	4.00
F	TMWW-t	MT20	4.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMW+w	MT20	2.0	4.0		
I	TTWW+m	MT20	5.0	6.0	2.50	1.50
J	TMWW-t	MT20	4.0	4.0	2.00	1.50
K	TMWW+t	MT20	4.0	6.0	2.75	1.25
L	TMW-t	MT20	4.0	6.0	1.50	3.00
N	BMV1+p	MT20	2.0	4.0		
O	BBWW-t	MT20	4.0	9.0	2.00	7.50
P	BBWW-t	MT20	7.0	8.0		
Q	BMWW-t	MT20	4.0	4.0		
R	BMWWW-t	MT20	5.0	6.0		
S	BBWW-t	MT20	10.0	12.0	Edge	
T	BBW-h	MT20	5.0	8.0	1.75	5.00
U	BMWWW-t	MT20	5.0	6.0		
V	BMVW1-t	MT20	4.0	6.0	1.75	3.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1550	0	1550	0	0	5-8	1-11
V	1546	0	1546	0	0	5-8	1-11

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1090	751 / 0	0 / 0	0 / 0	0 / 0	338 / 0	0 / 0
V	1087	749 / 0	0 / 0	0 / 0	0 / 0	338 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, V

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.81FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF E-U, E-T.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 27	-70.5	-70.5	0.09 (1)		10.00
B- C	0 / 16	-70.5	-70.5	0.16 (1)		10.00
C- D	-1684 / 0	-70.5	-70.5	0.20 (1)		4.93
D- E	-1396 / 0	-70.5	-70.5	0.59 (1)		4.75
E- F	-2267 / 0	-70.5	-70.5	0.54 (1)		3.81
F- G	-2133 / 0	-70.5	-70.5	0.32 (1)		4.35
G- H	-2133 / 0	-70.5	-70.5	0.32 (1)		4.35
H- I	-2133 / 0	-70.5	-70.5	0.32 (1)		4.35
I- J	-1970 / 0	-70.5	-70.5	0.30 (1)		4.55
J- K	-2270 / 0	-70.5	-70.5	0.26 (1)		4.27
K- L	-1302 / 0	-70.5	-70.5	0.11 (1)		5.54
L- M	0 / 27	-70.5	-70.5	0.09 (1)		10.00
V- B	-205 / 0	0.0	0.0	0.02 (1)		7.81
N- L	-1534 / 0	0.0	0.0	0.16 (1)		5.64
V- U	0 / 1363	-17.5	-17.5	0.38 (4)		10.00
U- T	0 / 1938	-17.5	-17.5	0.46 (1)		10.00
T- S	0 / 2697	-17.5	-17.5	0.32 (1)		10.00
S- R	0 / 2261	-17.5	-17.5	0.41 (1)		10.00
R- Q	0 / 1627	-17.5	-17.5	0.32 (1)		10.00
Q- P	0 / 1976	-17.5	-17.5	0.37 (1)		10.00
P- O	0 / 1377	-17.5	-17.5	0.22 (1)		10.00
O- N	0 / 0	-17.5	-17.5	0.02 (4)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- U	0 / 63	0.02 (4)
U- D	0 / 625	0.14 (1)
U- E	-745 / 0	0.45 (1)
T- E	-1830 / 0	0.52 (1)
E- S	0 / 1752	0.39 (1)
S- F	0 / 280	0.06 (1)
F- R	-198 / 0	0.18 (1)
R- H	-392 / 0	0.19 (1)
R- I	0 / 771	0.17 (1)
Q- I	0 / 228	0.05 (1)
Q- J	-381 / 0	0.19 (1)
P- J	-58 / 46	0.02 (4)
P- K	0 / 1478	0.33 (1)
O- K	-1550 / 0	0.25 (1)
V- C	-1872 / 0	0.89 (1)
O- L	0 / 1206	0.27 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	21.0	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.27")

CSI: TC=0.59 (D-E:1) , BC=0.46 (T-U:1) , WB=0.89 (C-V:1) , SSI=0.26 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (C) (INPUT = 0.90)
JSI METAL= 0.46 (C) (INPUT = 1.00)

A-15073832

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253630	H01T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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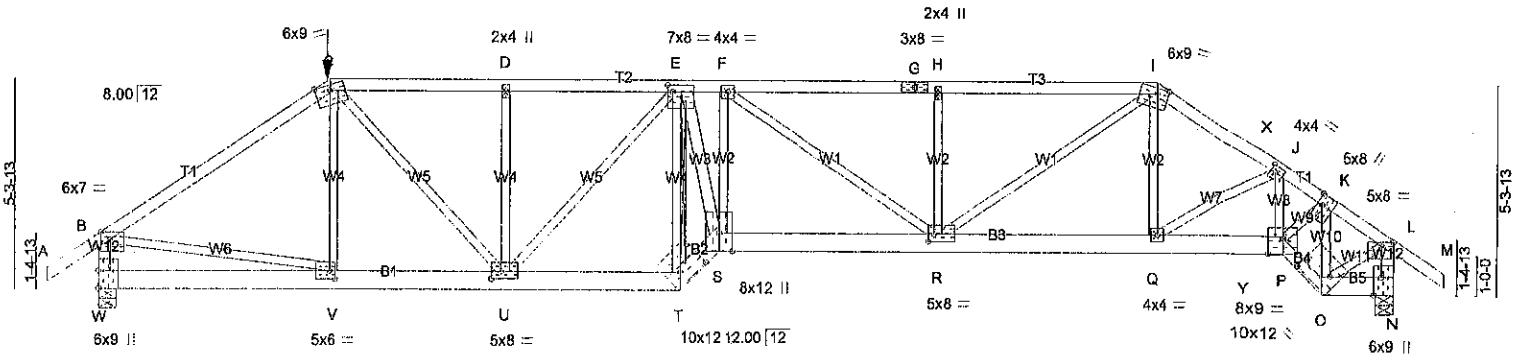

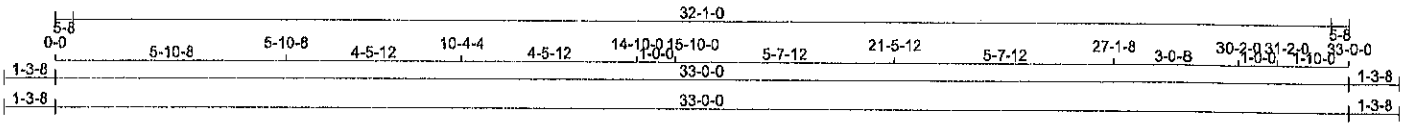

TOTAL WEIGHT = 164 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x4	DRY	1650F 1.5E	SPF
G - I	2x4	DRY	1650F 1.5E	SPF
I - M	2x4	DRY	No.2	SPF
W - B	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
W - T	2x6	DRY	No.2	SPF
T - S	2x6	DRY	No.2	SPF
S - P	2x6	DRY	1650F 1.5E	SPF
P - O	2x4	DRY	No.2	SPF
O - N	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
E - S	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	6.0	7.0	2.25	2.75
C	TTWW-m	MT20	6.0	9.0	Edge	
D	TMW+w	MT20	2.0	4.0		
E	TMWWW-t	MT20	7.0	8.0	2.00	1.50
F	TMWW-t	MT20	4.0	4.0		
G	TS-t	MT20	3.0	8.0		
H	TMW+w	MT20	2.0	4.0		
I	TTWW-m	MT20	6.0	9.0	1.75	3.50
J	TMWW-t	MT20	4.0	4.0	2.00	1.50
K	TMWW+t	MT20	5.0	8.0	3.25	1.50
L	TMVW-p	MT20	5.0	8.0	1.25	3.50
N	BMV1+t	MT20	6.0	9.0	Edge	2.50
O	BBWW-h	MT20	10.0	12.0	5.00	9.25
P	BBWW-t	MT20	8.0	9.0	4.75	4.50
Q	BMWW-t	MT20	4.0	4.0		
R	BMWWW-t	MT20	5.0	8.0	2.25	1.75
S	BBWW+p	MT20	8.0	12.0	Edge	
T	BBW-h	MT20	10.0	12.0	3.50	7.75
U	BMWWW-t	MT20	5.0	8.0	2.25	3.00
V	BMWW-t	MT20	5.0	6.0	2.50	1.75
W	BMV1+t	MT20	6.0	9.0	5.50	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
W	2838	0	2838	0	0	5-8	5-8
N	2977	0	2977	0	0	5-8	5-8

ALLOW FOR 0.3" OF HORIZONTAL MOVEMENT DUE TO TOTAL LOAD

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	2000	1352 / 0	0 / 0	0 / 0	0 / 0	648 / 0	0 / 0
N	2095	1435 / 0	0 / 0	0 / 0	0 / 0	660 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.92FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

2x4 DRY SPF No.2 I-BRACE REQUIRED AT E-T
FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH
A-B	0 / 27	-70.5	-70.5	0.10 (1)		10.00
B-C	-3522 / 0	-70.5	-70.5	1.00 (1)		2.93
C-D	-4228 / 0	-138.7	-138.7	0.51 (1)		3.45
D-E	-4228 / 0	-138.7	-138.7	0.51 (1)		3.45
E-F	-6137 / 0	-138.7	-138.7	0.40 (1)		2.97
F-G	-5592 / 0	-70.5	-70.5	0.64 (1)		3.03
G-H	-5592 / 0	-70.5	-70.5	0.64 (1)		3.03
H-I	-5592 / 0	-70.5	-70.5	0.66 (1)		3.03
I-X	-4322 / 0	-70.5	-70.5	0.38 (1)		3.06
X-J	-4322 / 0	-138.7	-138.7	0.38 (1)		3.06
J-K	-4649 / 0	-138.7	-138.7	0.37 (1)		2.92
K-L	-2661 / 0	-138.7	-138.7	0.21 (1)		4.03
L-M	0 / 27	-70.5	-70.5	0.10 (1)		10.00
W-B	-2750 / 0	0.0	0.0	0.31 (1)		5.13
N-L	-2946 / 0	0.0	0.0	0.33 (1)		4.96
W-V	0 / 0	-34.5	-34.5	0.14 (4)		10.00
V-U	0 / 2921	-34.5	-34.5	0.46 (1)		10.00
U-T	0 / 4861	-34.5	-34.5	0.74 (1)		10.00
T-S	0 / 6735	-34.5	-34.5	0.88 (1)		10.00
S-R	0 / 6131	-102.7	-102.7	0.66 (1)		10.00
R-Q	0 / 3694	-102.7	-102.7	0.40 (1)		10.00
Q-Y	0 / 3900	-102.7	-102.7	0.43 (1)		10.00
Y-P	0 / 3900	-34.5	-34.5	0.43 (1)		10.00
P-O	0 / 2863	-34.5	-34.5	0.51 (1)		10.00
O-N	0 / 0	-34.5	-34.5	0.02 (4)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V-C	-252 / 86	0.10 (1)
C-U	0 / 1942	0.48 (1)
U-D	-687 / 0	0.28 (1)
U-E	-954 / 0	0.74 (1)
T-E	-4595 / 0	0.96 (1)
E-S	0 / 5041	0.89 (1)
S-F	0 / 236	0.06 (1)
F-R	-662 / 0	0.56 (1)
R-H	-453 / 0	0.12 (1)
R-I	0 / 2433	0.60 (1)
Q-I	0 / 735	0.18 (1)
Q-J	-387 / 0	0.10 (1)
P-J	0 / 91	0.03 (4)
P-K	0 / 2728	0.68 (1)
O-K	-3176 / 0	0.54 (1)
B-V	0 / 2967	0.73 (1)
O-L	0 / 2485	0.62 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	5-10-8	-302	-302	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 5-10-8
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 15-10-0 OF SPAN MEASURED FROM THE LEFT.

GIRDER TYPE: CStdGirder
START DISTANCE = 15-10-0
START SPAN CARRIED = 5-10-8
END DISTANCE = 29-3-8
END SPAN CARRIED = 5-10-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 0-0
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 3-8-8 OF SPAN MEASURED FROM THE RIGHT.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.38")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 613 (0.65")

CSI: TC=1.00 (B-C:1) , BC=0.88 (S-T:1) , WB=0.96 (E-T:1) , SSI=0.32 (C-D:1)

A-401383

CONTINUED ON PAGE 2

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 301.8 lbs FACTORED DOWN AT 5-10-8 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (U) (INPUT = 0.90)
JSI METAL= 0.84 (E) (INPUT = 1.00)

A-4073831(2)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253629	H13	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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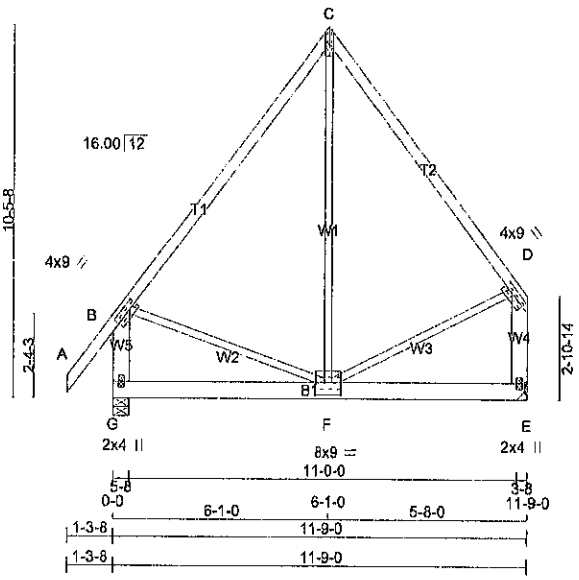
3x8 II

Scale = 1:65.1



TOTAL WEIGHT = 72 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	1650F 1.5E	SPF
C - D	2x4	DRY	No.2	SPF
G - B	2x6	DRY	No.2	SPF
E - D	2x6	DRY	No.2	SPF
G - E	2x6	DRY	1650F 1.5E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	9.0	2.00	3.00
C	TTW+p	MT20	3.0	8.0	3.75	Edge
D	TMVW-t	MT20	4.0	9.0	1.75	3.00
E	BMV1+p	MT20	2.0	4.0		
F	BMWWW-t	MT20	8.0	9.0	4.25	4.50
G	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	2521	0	2521	0	0	5-8	2-12
E	2422	0	2422	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 2-10	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1774	1215 / 0	0 / 0	0 / 0	0 / 0	559 / 0	0 / 0
E	1706	1166 / 0	0 / 0	0 / 0	0 / 0	551 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.80FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-70.5	-70.5	0.09 (1)	10.00	F- C	0 / 1850	0.46 (1)
B- C	-1415 / 0	-70.5	-70.5	0.59 (1)	5.44	B- F	0 / 889	0.22 (1)
C- D	-1415 / 0	-70.5	-70.5	0.67 (1)	4.80	F- D	0 / 924	0.23 (1)
G- B	-1706 / 0	0.0	0.0	0.16 (1)	7.55			
E- D	-1696 / 0	0.0	0.0	0.19 (1)	7.57			
G- F	0 / 0	-341.8	-341.8	0.70 (1)	10.00			
F- E	0 / 0	-341.8	-341.8	0.70 (1)	10.00			

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.39")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.39")
CALCULATED VERT. DEFL.(TL) = L/ 834 (0.17")

CSI: TC=0.67 (C-D:1) , BC=0.70 (F-G:1) , WB=0.46 (C-F:1) , SSI=0.84 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (G) (INPUT = 0.90)
JSI METAL= 0.34 (G) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07.17.2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

A-N073830

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253629	H12S	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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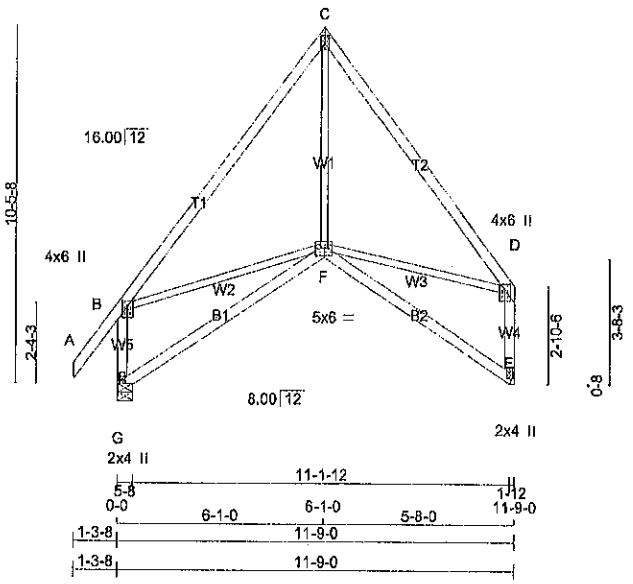

TOTAL WEIGHT = 2 X 61 = 123 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	1650F 1.5E	SPF
C - D	2x4	DRY	1650F 1.5E	SPF
G - B	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TTW+p	MT20	3.0	5.0	2.00	Edge
D	TMVW+p	MT20	4.0	6.0	2.00	2.00
E	BMV1+p	MT20	2.0	4.0		
F	BBWWW-p	MT20	5.0	6.0	2.75	3.00
G	BMV1+p	MT20	2.0	4.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	616	0	616	0	0	5-8	1-8
E	517	0	517	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	432	306 / 0	0 / 0	0 / 0	0 / 0	126 / 0	0 / 0
E	364	247 / 0	0 / 0	0 / 0	0 / 0	118 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-70.5	-70.5	0.08 (1)	10.00	F- C	0 / 226	0.05 (4)
B- C	-400 / 0	-70.5	-70.5	0.28 (1)	6.25	B- F	0 / 260	0.06 (1)
C- D	-400 / 0	-70.5	-70.5	0.25 (1)	6.25	F- D	0 / 246	0.06 (1)
G- B	-563 / 0	0.0	0.0	0.08 (1)	7.81			
E- D	-467 / 0	0.0	0.0	0.07 (1)	7.81			
G- F	0 / 0	-17.5	-17.5	0.19 (4)	10.00			
F- E	0 / 0	-17.5	-17.5	0.16 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.39")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.39")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.28 (B-C:1) , BC=0.19 (F-G:4) , WB=0.06 (B-F:1) , SSI=0.10 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.71 (G) (INPUT = 0.90)
JSI METAL= 0.13 (G) (INPUT = 1.00)

A-150738 29

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253629	H12	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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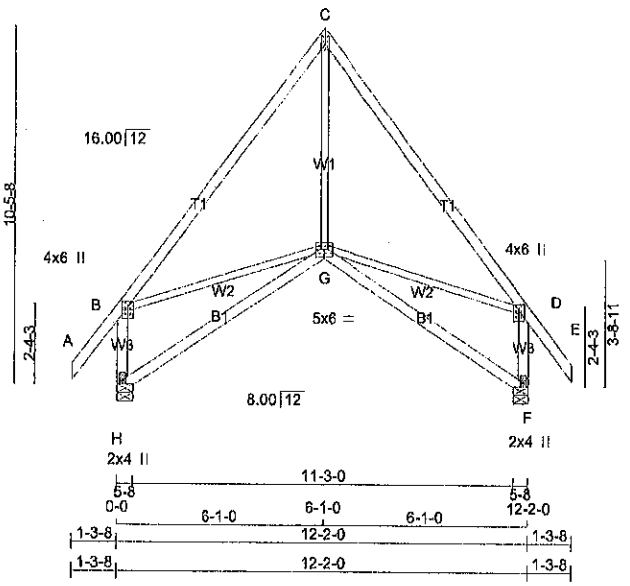
ID:AyrJx9Y7fa2EXF7DRQxSq4zMEz1-YtleYZc9KBg5cRrr7egVmLeZ8FOILvvVJdWBmpyxE5W

3x5 II

Scale = 1:67.9



TOTAL WEIGHT = 65 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	1650F 1.5E	SPF
C - E	2x4	DRY	1650F 1.5E	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TTW+p	MT20	3.0	5.0	2.00	Edge
D	TMVW+p	MT20	4.0	6.0	2.00	2.00
F	BMV1+p	MT20	2.0	4.0	Edge	
G	BBWWW-p	MT20	5.0	6.0	2.75	3.00
H	BMV1+p	MT20	2.0	4.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	634	0	634	0	0	5-8	1-8
F	634	0	634	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	445	314 / 0	0 / 0	0 / 0	0 / 0	130 / 0	0 / 0
F	445	314 / 0	0 / 0	0 / 0	0 / 0	130 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 40	-70.5	-70.5	0.06 (1)	10.00	G-C	0 / 268	0.06 (1)
B-C	-429 / 0	-70.5	-70.5	0.29 (1)	6.25	B-G	0 / 268	0.06 (1)
C-D	-429 / 0	-70.5	-70.5	0.29 (1)	6.25	G-D	0 / 268	0.06 (1)
D-E	0 / 40	-70.5	-70.5	0.06 (1)	10.00			
H-B	-581 / 0	0.0	0.0	0.06 (1)	7.81			
F-D	-581 / 0	0.0	0.0	0.06 (1)	7.81			
H-G	0 / 0	-17.5	-17.5	0.19 (4)	10.00			
G-F	0 / 0	-17.5	-17.5	0.19 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.29 (C-D:1) , BC=0.19 (G-H:4) , WB=0.06 (D-G:1) , SSI=0.10 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.60

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (H) (INPUT = 0.90)
JSI METAL= 0.14 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07.17.2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

A-N073828

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253629	H11G	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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ID:AyrJx9Y?fa2EXF7DRQxSq4zMEz1-4hkGKDbXZtYE_HGfZx9GE76RGr4FcQHL4zndENyxE5

3x4 II

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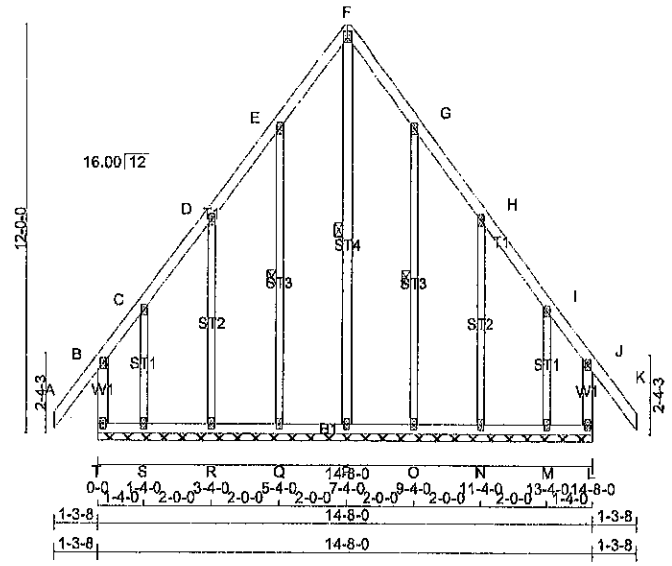

TOTAL WEIGHT = 98 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - F	2x4	DRY	No.2	SPF
F - K	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
P - F	2x4	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
EXCEPT				
ST1	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C, D, E, G, H, I						
C	TMW+w	MT20	2.0	4.0		
F	TTW+p	MT20	3.0	4.0	1.75	1.50
J	TMV+p	MT20	2.0	4.0		
L	BMV1+p	MT20	2.0	4.0		
M, N, O, P, Q, R, S						
M	BMW1+w	MT20	2.0	4.0		
T	BMV1+p	MT20	2.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF F-P, E-Q, G-O.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 40	-70.5	-70.5	0.10 (1)	10.00
B- C	-10 / 0	-70.5	-70.5	0.07 (1)	10.00
C- D	0 / 25	-70.5	-70.5	0.04 (1)	10.00
D- E	0 / 23	-70.5	-70.5	0.04 (1)	10.00
E- F	0 / 27	-70.5	-70.5	0.04 (1)	10.00
F- G	0 / 27	-70.5	-70.5	0.04 (1)	10.00
G- H	0 / 23	-70.5	-70.5	0.04 (1)	10.00
H- I	0 / 25	-70.5	-70.5	0.04 (1)	10.00
I- J	-10 / 0	-70.5	-70.5	0.07 (1)	10.00
J- K	0 / 40	-70.5	-70.5	0.10 (1)	10.00
T- B	-161 / 0	0.0	0.0	0.03 (1)	7.81
L- J	-161 / 0	0.0	0.0	0.03 (1)	7.81
T- S	-9 / 0	-17.5	-17.5	0.01 (4)	10.00
S- R	-11 / 0	-17.5	-17.5	0.01 (4)	6.25
R- Q	-14 / 0	-17.5	-17.5	0.01 (4)	6.25
Q- P	-16 / 0	-17.5	-17.5	0.01 (4)	6.25
P- O	-16 / 0	-17.5	-17.5	0.01 (4)	6.25
O- N	-14 / 0	-17.5	-17.5	0.01 (4)	6.25
N- M	-11 / 0	-17.5	-17.5	0.01 (4)	6.25
M- L	-9 / 0	-17.5	-17.5	0.01 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- F	-192 / 0	0.15 (1)
Q- E	-141 / 0	0.08 (1)
R- D	-145 / 0	0.11 (1)
S- C	-72 / 0	0.02 (1)
O- G	-141 / 0	0.08 (1)
N- H	-146 / 0	0.11 (1)
M- I	-72 / 0	0.02 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.10 (A-B:1) , BC=0.01 (Q-R:4) , WB=0.15 (F-P:1) , SSI=0.05 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	364	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.29 (F) (INPUT = 0.90)
JSI METAL= 0.06 (F) (INPUT = 1.00)

A-ND73827

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253629	H11A	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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ID:AyrJx9Y?fa2EXF7DRQxSq4zMEz1-4hkGKDbXZtYE_HGfZx9GE76NkrxLcKpL4zndENyxE5)

3x8 II

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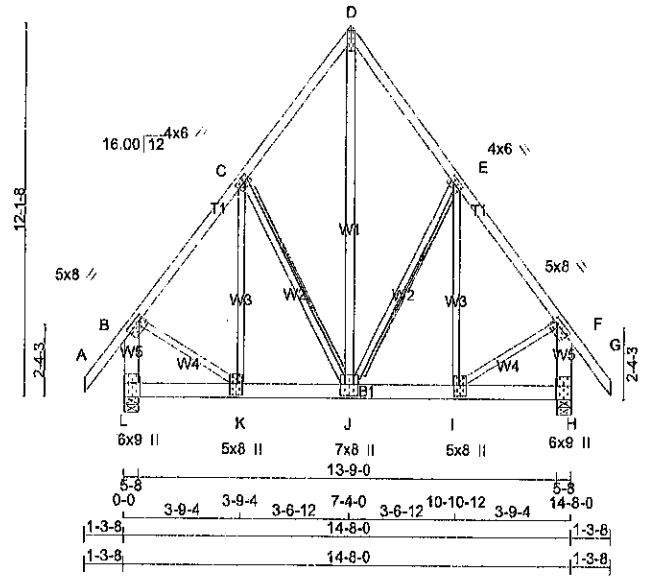


TOTAL WEIGHT = 109 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
L - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
J - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	1.75	2.00
C	TMWW-t	MT20	4.0	6.0	2.00	2.25
D	TTW+p	MT20	3.0	8.0	3.75	Edge
E	TMWW-t	MT20	4.0	6.0	2.00	2.25
F	TMVW-t	MT20	5.0	8.0	1.75	2.00
H	BMV1+t	MT20	6.0	9.0	Edge	0.50
I	BMWW+t	MT20	5.0	8.0	4.25	2.00
J	BMWWW+t	MT20	7.0	8.0	4.25	3.50
K	BMWW+t	MT20	5.0	8.0	4.25	2.00
L	BMV1+t	MT20	6.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	4012	0	4012	0	0	5-8	5-8
H	4012	0	4012	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	2825	1926 / 0	0 / 0	0 / 0	0 / 0	898 / 0	0 / 0
H	2825	1926 / 0	0 / 0	0 / 0	0 / 0	898 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.93FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

2x4 DRY SPF No.2 T-BRACE REQUIRED AT E-J, C-J
FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX UNBRAC LENGTH
A-B	0 / 40	-70.5	-70.5	0.11 (1)		10.00
B-C	-2769 / 0	-70.5	-70.5	0.33 (1)		3.93
C-D	-2136 / 0	-70.5	-70.5	0.24 (1)		4.44
D-E	-2136 / 0	-70.5	-70.5	0.24 (1)		4.44
E-F	-2769 / 0	-70.5	-70.5	0.33 (1)		3.93
F-G	0 / 40	-70.5	-70.5	0.11 (1)		10.00
L-B	-3272 / 0	0.0	0.0	0.29 (1)		5.81
H-F	-3272 / 0	0.0	0.0	0.29 (1)		5.81
L-K	0 / 0	-463.2	-463.2	0.44 (1)		10.00
K-J	0 / 1673	-463.2	-463.2	0.58 (1)		10.00
J-I	0 / 1673	-463.2	-463.2	0.58 (1)		10.00
I-H	0 / 0	-463.2	-463.2	0.44 (1)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI
J-D	0 / 3184	0.56 (1)
J-E	-879 / 0	0.41 (1)
I-E	0 / 1012	0.25 (1)
C-J	-879 / 0	0.41 (1)
K-C	0 / 1012	0.25 (1)
B-K	0 / 1868	0.46 (1)
I-F	0 / 1868	0.46 (1)

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(65 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.49")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.49")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.33 (E-F:1) , BC=0.58 (J-K:1) , WB=0.56 (D-J:1) , SSI=0.67 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (F) (INPUT = 0.90)
JSI METAL= 0.59 (D) (INPUT = 1.00)

A-10073826

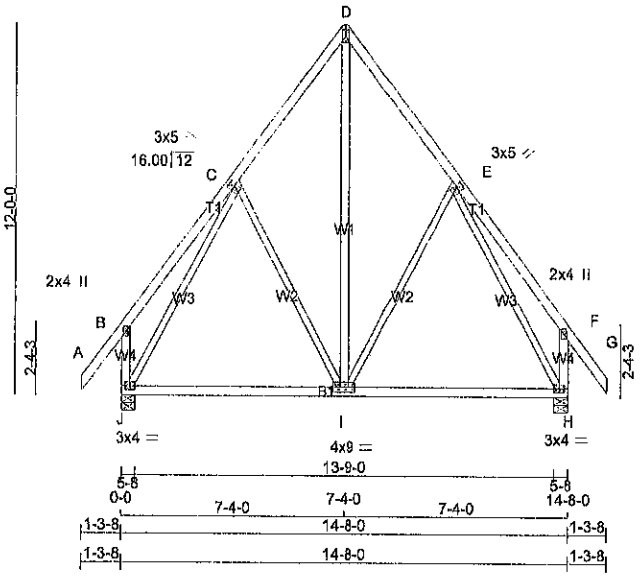
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253629	H11	3	1	TRUSS DESC.	

Alpa Roof Truss, Maple

ID:AyrJx9Y?fa2EXF7DRQxSq4zMEz1-cVBu7tavoZQNM7hT?Ee1hwZFhSgltrqCrJ14lxyxE5\

3x5 II

Scale = 1:75.1



TOTAL WEIGHT = 3 X 92 = 275 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
I - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWW+t	MT20	3.0	5.0	2.25	0.75
D	TTW+p	MT20	3.0	5.0		
E	TMWW+t	MT20	3.0	5.0	2.25	0.75
F	TMV+p	MT20	2.0	4.0		
H	BMVW1-t	MT20	3.0	4.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BMVW1-t	MT20	3.0	4.0		

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	744	0	744	0	0	5-8	1-8
H	744	0	744	0	0	6-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	522	367 / 0	0 / 0	0 / 0	0 / 0	155 / 0	0 / 0
H	522	367 / 0	0 / 0	0 / 0	0 / 0	155 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-70.5	-70.5	0.10 (1)	10.00	I- D	0 / 359	0.06 (1)
B- C	0 / 25	-70.5	-70.5	0.16 (1)	10.00	I- E	-115 / 2	0.14 (1)
C- D	-376 / 0	-70.5	-70.5	0.12 (1)	6.25	C- I	-115 / 2	0.14 (1)
D- E	-376 / 0	-70.5	-70.5	0.12 (1)	6.25	J- C	-567 / 0	0.67 (1)
E- F	0 / 25	-70.5	-70.5	0.16 (1)	10.00	E- H	-567 / 0	0.67 (1)
F- G	0 / 40	-70.5	-70.5	0.10 (1)	10.00			
J- B	-195 / 0	0.0	0.0	0.02 (1)	7.81			
H- F	-195 / 0	0.0	0.0	0.02 (1)	7.81			
J- I	0 / 266	-17.5	-17.5	0.29 (4)	10.00			
I- H	0 / 266	-17.5	-17.5	0.29 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.49")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.49")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.16 (B-C:1) , BC=0.29 (H-I:4) , WB=0.67 (E-H:1) , SSI=0.11 (I-J:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.60

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.73 (C) (INPUT = 0.90)
JSI METAL= 0.28 (C) (INPUT = 1.00)

A-1507382C

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253629	H10G	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

ID:AyrJx9Y?fa2EXF7DRQxSq4zMEz1-8IdWwXZH1GIWlz6HSW7o8i06m2OI8Yp3dfIWAVyxE5Z

3x5 II

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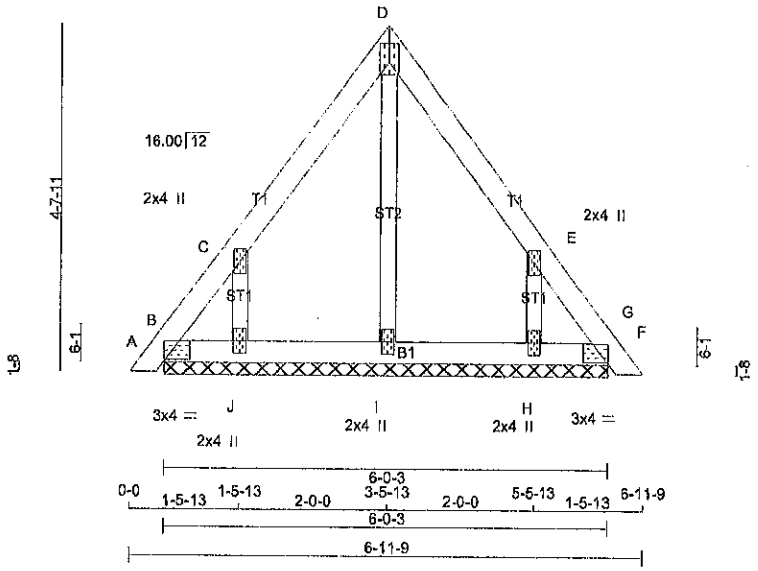

TOTAL WEIGHT = 2 X 25 = 50 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.75
C	TMW+w	MT20	2.0	4.0		
D	TTW+p	MT20	3.0	5.0	2.00	Edge
E	TMW+w	MT20	2.0	4.0		
F	TMB1-I	MT20	3.0	4.0	1.50	2.75
H, I, J						
H	BMW1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 9	-70.5	-70.5	0.01 (1)	10.00	I- D	-59 / 0	0.02 (1)
B- C	-37 / 0	-70.5	-70.5	0.04 (1)	6.25	J- C	-132 / 0	0.02 (1)
C- D	-50 / 0	-70.5	-70.5	0.04 (1)	6.25	H- E	-132 / 0	0.02 (1)
D- E	-50 / 0	-70.5	-70.5	0.04 (1)	6.25			
E- F	-37 / 0	-70.5	-70.5	0.04 (1)	6.25			
F- G	0 / 9	-70.5	-70.5	0.01 (1)	10.00			
B- J	0 / 36	-17.5	-17.5	0.01 (1)	10.00			
J- I	0 / 24	-17.5	-17.5	0.02 (4)	10.00			
I- H	0 / 24	-17.5	-17.5	0.02 (4)	10.00			
H- F	0 / 36	-17.5	-17.5	0.01 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.04 (C-D:1) , BC=0.02 (I-J:4) , WB=0.02 (E-H:1) , SSI=0.04 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (B) (INPUT = 0.90)
JSI METAL= 0.04 (E) (INPUT = 1.00)

A-N073824

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253629	H10	17	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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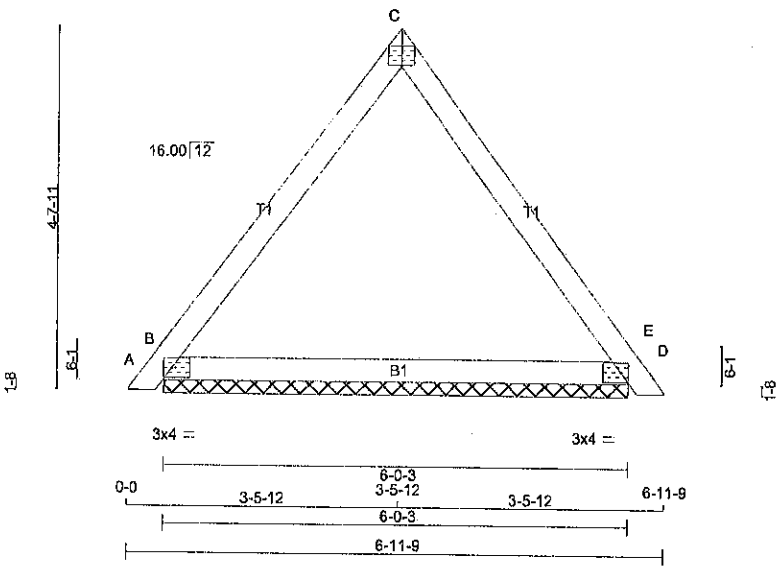

TOTAL WEIGHT = 17 X 20 = 341 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.75
C	TT-p	MT20	3.0	4.0	3.25	Edge
D	TMB1-I	MT20	3.0	4.0	1.50	2.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	289	0	289	0	0	6-0-3	1-8
D	289	0	289	0	0	6-0-3	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	203	141 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0
D	203	141 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH
A-B	0 / 9	-70.5	-70.5	0.01 (1)	10.00
B-C	-131 / 0	-70.5	-70.5	0.11 (1)	6.25
C-D	-131 / 0	-70.5	-70.5	0.11 (1)	6.25
D-E	0 / 9	-70.5	-70.5	0.01 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B-D	0 / 77	-17.5 -17.5 0.19 (4) 10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11 (C-D:1) , BC=0.19 (B-D:4) , WB=0.00 (n/a:0) , SSI=0.07 (B-D:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.45 (D) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
@ 2.17.2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

A-1507382 3

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253629	H09G	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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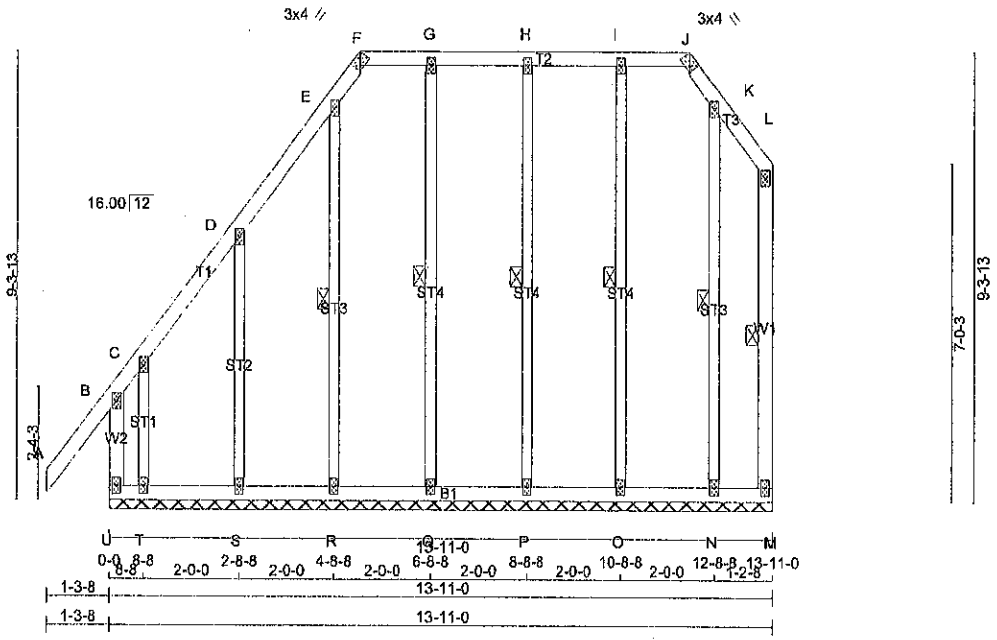

TOTAL WEIGHT = 91 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - F	2x4	DRY	No.2	SPF
F - J	2x4	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
U - M	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C, D, E, G, H, I, K						
C	TMW+w	MT20	2.0	4.0		
F	TT-h	MT20	3.0	4.0	Edge	1.25
J	TT-h	MT20	3.0	4.0	Edge	1.25
L	TMV+p	MT20	2.0	4.0		
M	BMV1+p	MT20	2.0	4.0		
N, O, P, Q, R, S, T						
N	BMW1+w	MT20	2.0	4.0		
U	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF L-M, H-P, G-Q, E-R, I-O, K-N.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 40	-70.5	-70.5	0.10 (1)	10.00
B- C	-73 / 0	-70.5	-70.5	0.09 (1)	6.25
C- D	-4 / 0	-70.5	-70.5	0.03 (1)	10.00
D- E	-3 / 0	-70.5	-70.5	0.03 (1)	10.00
E- F	-44 / 0	-70.5	-70.5	0.03 (1)	6.25
F- G	0 / 1	-78.0	-78.0	0.04 (1)	10.00
G- H	0 / 0	-78.0	-78.0	0.04 (1)	10.00
H- I	0 / 0	-78.0	-78.0	0.04 (1)	10.00
I- J	0 / 2	-78.0	-78.0	0.04 (1)	10.00
J- K	-40 / 0	-70.5	-70.5	0.02 (1)	6.25
K- L	0 / 6	-70.5	-70.5	0.02 (1)	10.00
U- B	-208 / 0	0.0	0.0	0.02 (1)	7.81
M- L	-30 / 0	0.0	0.0	0.01 (1)	6.25
U- T	0 / 5	-17.5	-17.5	0.02 (1)	10.00
T- S	0 / 5	-17.5	-17.5	0.01 (4)	10.00
S- R	0 / 2	-17.5	-17.5	0.01 (4)	10.00
R- Q	0 / 0	-17.5	-17.5	0.01 (4)	10.00
Q- P	0 / 0	-17.5	-17.5	0.01 (4)	10.00
P- O	0 / 0	-17.5	-17.5	0.01 (4)	10.00
O- N	0 / 0	-17.5	-17.5	0.01 (4)	10.00
N- M	0 / 1	-17.5	-17.5	0.01 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- H	-153 / 0	0.09 (1)
Q- G	-163 / 0	0.09 (1)
R- E	-144 / 0	0.07 (1)
S- D	-142 / 0	0.08 (1)
T- C	-5 / 0	0.00 (1)
O- I	-168 / 0	0.10 (1)
N- K	-119 / 0	0.06 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.10 (A-B:1) , BC=0.02 (T-U:1) , WB=0.10 (I-O:1) , SSI=0.07 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

A-1073822

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253629	H09	6	1	TRUSS DESC.	

Alpa Roof Truss, Maple

ID:AyrJx9Y?fa2EXF7DRQxSq4zMEz1-g637iBYfGyAf7pX4upcZcVUpBe0xP3cvO?Yzd2yxE5i

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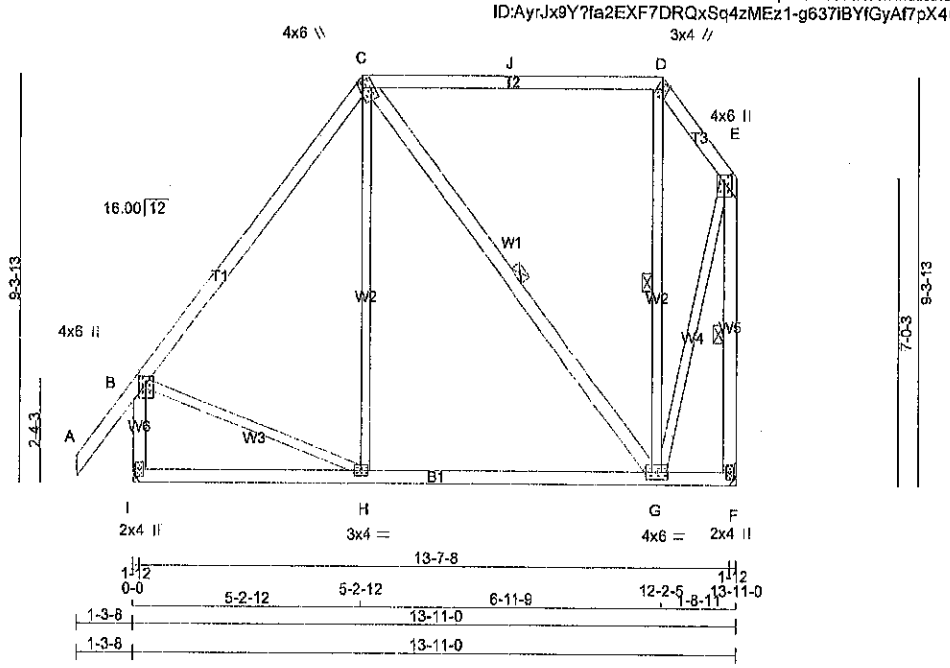

TOTAL WEIGHT = 6 X 87 = 521 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	1650F 1.5E	SPF
D - E	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TTWW+m	MT20	4.0	6.0	1.75	1.00
D	TTW+m	MT20	3.0	4.0	Edge	
E	TMVW+p	MT20	4.0	6.0	2.00	2.00
F	BMV1+p	MT20	2.0	4.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BMWW-t	MT20	3.0	4.0		
I	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	731	0	731	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
F	645	0	645	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	514	351 / 0	0 / 0	0 / 0	0 / 0	163 / 0	0 / 0
F	457	292 / 0	0 / 0	0 / 0	0 / 0	165 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF C-G, D-G, E-F.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF) CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-70.5	-70.5	0.10 (1)	10.00	H- C	0 / 102	0.04 (4)
B- C	-405 / 0	-70.5	-70.5	0.25 (1)	6.25	C- G	-206 / 0	0.14 (1)
C- J	-117 / 0	-78.0	-78.0	0.54 (1)	6.25	G- D	-170 / 0	0.10 (1)
J- D	-117 / 0	-78.0	-78.0	0.54 (1)	6.25	B- H	0 / 258	0.06 (1)
D- E	-201 / 0	-70.5	-70.5	0.03 (1)	6.25	G- E	0 / 447	0.10 (1)
I- B	-697 / 0	0.0	0.0	0.09 (1)	7.81			
F- E	-653 / 0	0.0	0.0	0.15 (1)	6.25			
I- H	0 / 0	-17.5	-17.5	0.16 (4)	10.00			
H- G	0 / 242	-17.5	-17.5	0.18 (4)	10.00			
G- F	0 / 0	-17.5	-17.5	0.12 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.46")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.46")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.64 (C-D:1) , BC=0.18 (G-H:4) , WB=0.14 (C-G:1) , SSI=0.21 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (G) (INPUT = 0.90)
JSI METAL= 0.13 (G) (INPUT = 1.00)

A-1507382/

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253629	H08G	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MiTek Industries, Inc. Fri Jul 17 08:27:04 2015 Page 1
ID:AyrJx9Y?fa2EXF7DRQxSq4zMEz1-CwVlVrY1Ve2oVgyuK55K3HxmHElJgd7m9LpQ5cyxE5t

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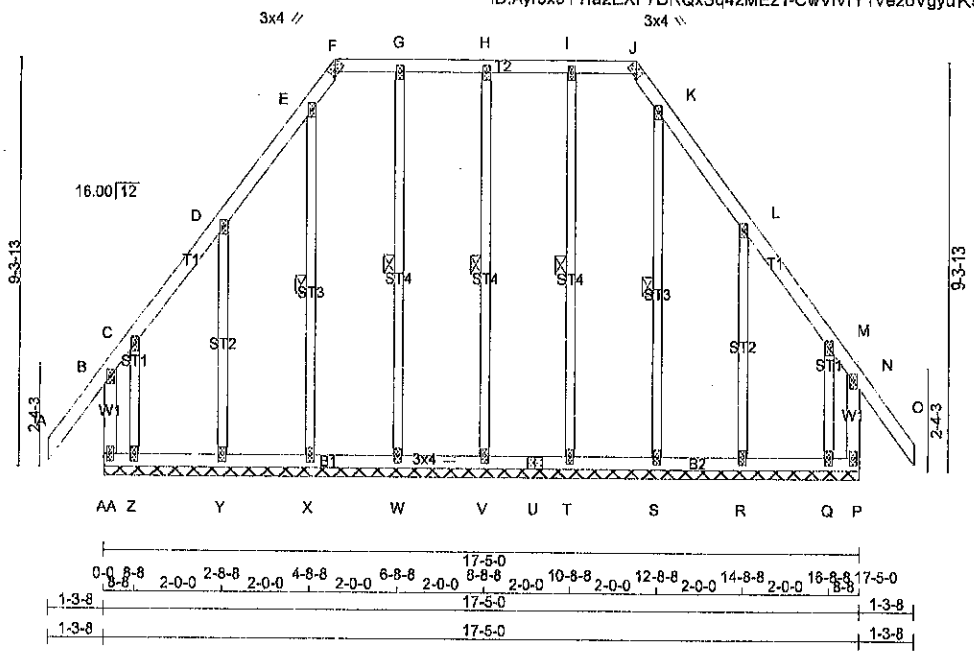


TOTAL WEIGHT = 105 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - F	2x4	DRY	No.2	SPF
F - J	2x4	DRY	No.2	SPF
J - O	2x4	DRY	No.2	SPF
AA- B	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
AA- U	2x4	DRY	No.2	SPF
U - P	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C, D, E, G, H, I, K, L, M						
C	TMW+w	MT20	2.0	4.0		
F	TT-h	MT20	3.0	4.0	Edge	1.25
J	TT-h	MT20	3.0	4.0	Edge	1.25
N	TMV+p	MT20	2.0	4.0		
P	BMV1+p	MT20	2.0	4.0		
Q, R, S, T, V, W, X, Y, Z						
Q	BMW1+w	MT20	2.0	4.0		
U	BS-t	MT20	3.0	4.0		
AA	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF H-V, G-W, E-X, I-T, K-S.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 40	-70.5	-70.5	0.10 (1)	10.00
B- C	-61 / 0	-70.5	-70.5	0.08 (1)	6.25
C- D	0 / 9	-70.5	-70.5	0.03 (1)	10.00
D- E	0 / 11	-70.5	-70.5	0.04 (1)	10.00
E- F	-36 / 0	-70.5	-70.5	0.03 (1)	6.25
F- G	0 / 10	-78.0	-78.0	0.04 (1)	10.00
G- H	0 / 9	-78.0	-78.0	0.04 (1)	10.00
H- I	0 / 9	-78.0	-78.0	0.04 (1)	10.00
I- J	0 / 10	-78.0	-78.0	0.04 (1)	10.00
J- K	-36 / 0	-70.5	-70.5	0.03 (1)	6.25
K- L	0 / 11	-70.5	-70.5	0.04 (1)	10.00
L- M	0 / 9	-70.5	-70.5	0.03 (1)	10.00
M- N	-51 / 0	-70.5	-70.5	0.06 (1)	6.25
N- O	0 / 40	-70.5	-70.5	0.10 (1)	10.00
AA- B	-186 / 0	0.0	0.0	0.02 (1)	7.81
P- N	-186 / 0	0.0	0.0	0.02 (1)	7.81
AA- Z	-3 / 0	-17.5	-17.5	0.01 (1)	10.00
Z- Y	-3 / 0	-17.5	-17.5	0.01 (4)	10.00
Y- X	-6 / 0	-17.5	-17.5	0.01 (4)	10.00
X- W	-9 / 0	-17.5	-17.5	0.01 (4)	10.00
W- V	-9 / 0	-17.5	-17.5	0.01 (4)	10.00
V- U	-9 / 0	-17.5	-17.5	0.01 (4)	10.00
U- T	-9 / 0	-17.5	-17.5	0.01 (4)	10.00
T- S	-9 / 0	-17.5	-17.5	0.01 (4)	10.00
S- R	-6 / 0	-17.5	-17.5	0.01 (4)	10.00
R- Q	-3 / 0	-17.5	-17.5	0.01 (4)	10.00
Q- P	-3 / 0	-17.5	-17.5	0.01 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V- H	-152 / 0	0.09 (1)
W- G	-167 / 0	0.10 (1)
X- E	-152 / 0	0.07 (1)
Y- D	-141 / 0	0.08 (1)
Z- C	-17 / 0	0.00 (1)
T- I	-167 / 0	0.10 (1)
S- K	-152 / 0	0.07 (1)
R- L	-141 / 0	0.08 (1)
Q- M	-17 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH:	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(65 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.10 (A-B:1) , BC=0.01 (X-Y:4) , WB=0.10 (G-W:1) , SSI=0.07 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.46 (F) (INPUT = 0.90)
JSI METAL= 0.05 (N) (INPUT = 1.00)

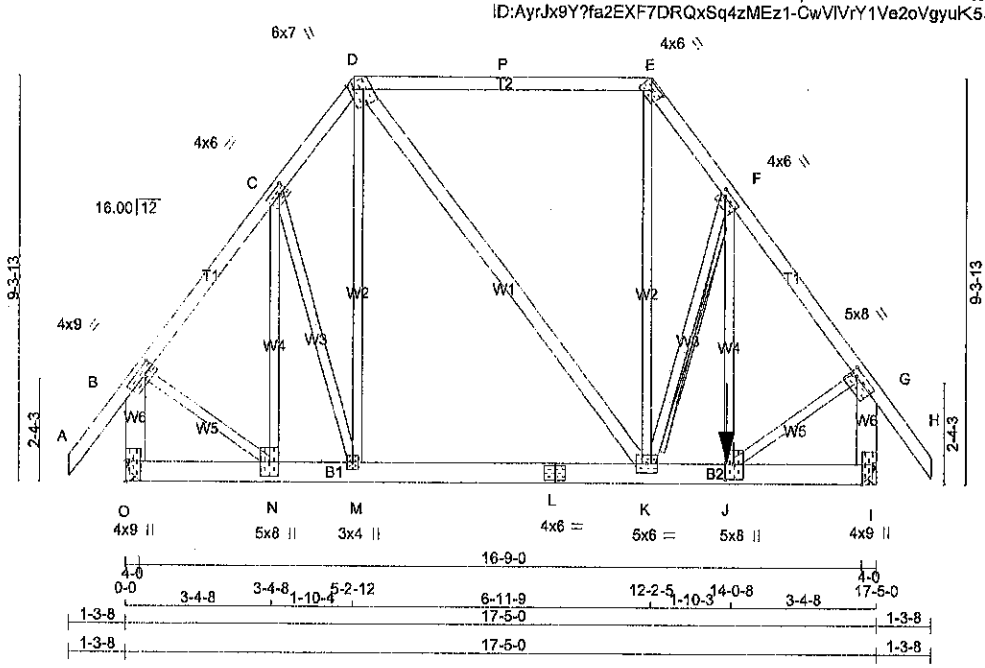
A-H073820

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253629	H08A	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

ID:AyrJx9Y?fa2EXF7DRQxSq4zMEz1-CwVlVrY1Ve2oVgyuK55K3HxdcEd3gWLm9LpQ5cyxE5t

Scale = 1:53.4



TOTAL WEIGHT = 124 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	1650F 1.5E	SPF
E - H	2x4	DRY	No.2	SPF
O - B	2x6	DRY	No.2	SPF
I - G	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	9.0	2.00	3.00
C	TMWW-t	MT20	4.0	6.0	2.00	2.50
D	TTWW+m	MT20	6.0	7.0	Edge	1.50
E	TTW-h	MT20	4.0	6.0	1.50	3.00
F	TMWW-t	MT20	4.0	6.0	1.50	1.50
G	TMVW-t	MT20	5.0	8.0	1.75	2.00
I	BMV1+t	MT20	4.0	9.0	Edge	1.50
J	BMWW+t	MT20	5.0	8.0	4.25	2.25
K	BMWWW-t	MT20	5.0	6.0		
L	BS-t	MT20	4.0	6.0		
M	BMWW+t	MT20	3.0	4.0		
N	BMWW+t	MT20	5.0	8.0		
O	BMV1+p	MT20	4.0	9.0	Edge	0.25

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 2421.3 lbs FACTORED DOWN AT 13-11-2 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1442	0	1442	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-9	
I	3499	0	3499	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 4-0	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1016	687 / 0	0 / 0	0 / 0	0 / 0	329 / 0	0 / 0
I	2465	1669 / 0	0 / 0	0 / 0	0 / 0	796 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.13FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

2x4 DRY SPF No.2 T-BRACE REQUIRED AT F-K
FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 40	-70.5	-70.5	0.11 (1)	10.00	N- C	-298 / 0		0.23 (1)
B- C	-1061 / 0	-70.5	-70.5	0.14 (1)	5.96	C- M	0 / 34		0.01 (4)
C- D	-1136 / 0	-70.5	-70.5	0.13 (1)	5.79	M- D	0 / 60		0.02 (4)
D- P	-1100 / 0	-78.0	-78.0	0.66 (1)	5.53	D- K	0 / 730		0.13 (1)
P- E	-1100 / 0	-78.0	-78.0	0.66 (1)	5.53	K- E	0 / 1161		0.29 (1)
E- F	-1848 / 0	-70.5	-70.5	0.09 (1)	4.84	K- F	-1487 / 0		0.53 (1)
F- G	-2500 / 0	-70.5	-70.5	0.28 (1)	4.13	J- F	0 / 1745		0.43 (1)
G- H	0 / 40	-70.5	-70.5	0.11 (1)	10.00	B- N	0 / 736		0.18 (1)
O- B	-1414 / 0	0.0	0.0	0.13 (1)	7.81	J- G	0 / 1714		0.42 (1)
I- G	-3039 / 0	0.0	0.0	0.27 (1)	6.00				
O- N	0 / 0	-17.5	-17.5	0.03 (4)	10.00				
N- M	0 / 646	-17.5	-17.5	0.09 (4)	10.00				
M- L	0 / 654	-17.5	-17.5	0.22 (1)	10.00				
L- K	0 / 654	-17.5	-17.5	0.22 (1)	10.00				
K- J	0 / 1505	-17.5	-17.5	0.35 (1)	10.00				
J- I	0 / 0	-231.9	-231.9	0.34 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
J	13-11-2	-2421	-2421	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

GIRDER TYPE: CStdGirder
START DISTANCE = 13-11-12
START SPAN CARRIED = 11-9-0
END DISTANCE = 17-5-0
END SPAN CARRIED = 11-9-0
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.58")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.58")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.66 (D-E:1) , BC=0.35 (J-K:1) , WB=0.53 (F-K:1) , SSI=0.31 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

A-ND73819

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253629	H08A	1	1	TRUSS DESC.	

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (J) (INPUT = 0.90)
JSI METAL= 0.50 (G) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
07.17.2015
R. TURENNE
100157040
PROVINCE OF ONTARIO

A-N073819(2)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253629	H08	10	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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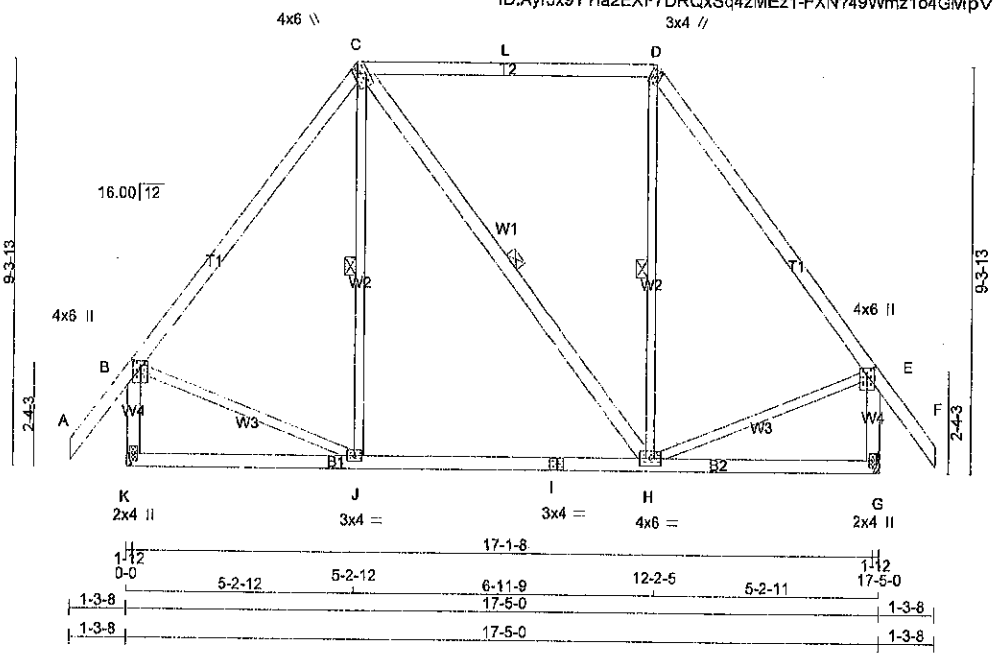

TOTAL WEIGHT = 10 X 93 = 933 lb
(M)(F)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	1650F 1.5E	SPF
D - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TTWW+m	MT20	4.0	6.0	1.75	1.00
D	TTW+m	MT20	3.0	4.0	1.75	1.00
E	TMVW+p	MT20	4.0	6.0	2.00	2.00
G	BMV1+p	MT20	2.0	4.0		
H	BMWWW-t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	4.0		
J	BMWW-t	MT20	3.0	4.0		
K	BMV1+p	MT20	2.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	891	0	891	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
G	891	0	891	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	628	425 / 0	0 / 0	0 / 0	0 / 0	204 / 0	0 / 0
G	628	425 / 0	0 / 0	0 / 0	0 / 0	204 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF C-J, C-H, D-H.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-70.5	-70.5	1	0.10 (1)	10.00	J- C	-6 / 86	0.03 (4)
B- C	-561 / 0	-70.5	-70.5	1	0.26 (1)	6.25	C- H	0 / 0	0.00 (1)
C- L	-335 / 0	-78.0	-78.0	1	0.54 (1)	6.25	H- D	-6 / 86	0.03 (4)
L- D	-335 / 0	-78.0	-78.0	1	0.54 (1)	6.25	B- J	0 / 357	0.08 (1)
D- E	-561 / 0	-70.5	-70.5	1	0.26 (1)	6.25	H- E	0 / 357	0.08 (1)
E- F	0 / 40	-70.5	-70.5	1	0.10 (1)	10.00			
K- B	-856 / 0	0.0	0.0	1	0.11 (1)	7.81			
G- E	-856 / 0	0.0	0.0	1	0.11 (1)	7.81			
K- J	0 / 0	-17.5	-17.5	4	0.15 (4)	10.00			
J- I	0 / 335	-17.5	-17.5	4	0.18 (4)	10.00			
I- H	0 / 335	-17.5	-17.5	4	0.18 (4)	10.00			
H- G	0 / 0	-17.5	-17.5	4	0.15 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -8.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.58")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.58")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.54 (C-D:1) , BC=0.18 (H-J:4) , WB=0.08 (B-J:1) , SSI=0.21 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.60

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (H) (INPUT = 0.90)
JSI METAL= 0.16 (G) (INPUT = 1.00)

A-150738 18

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253629	H07	9	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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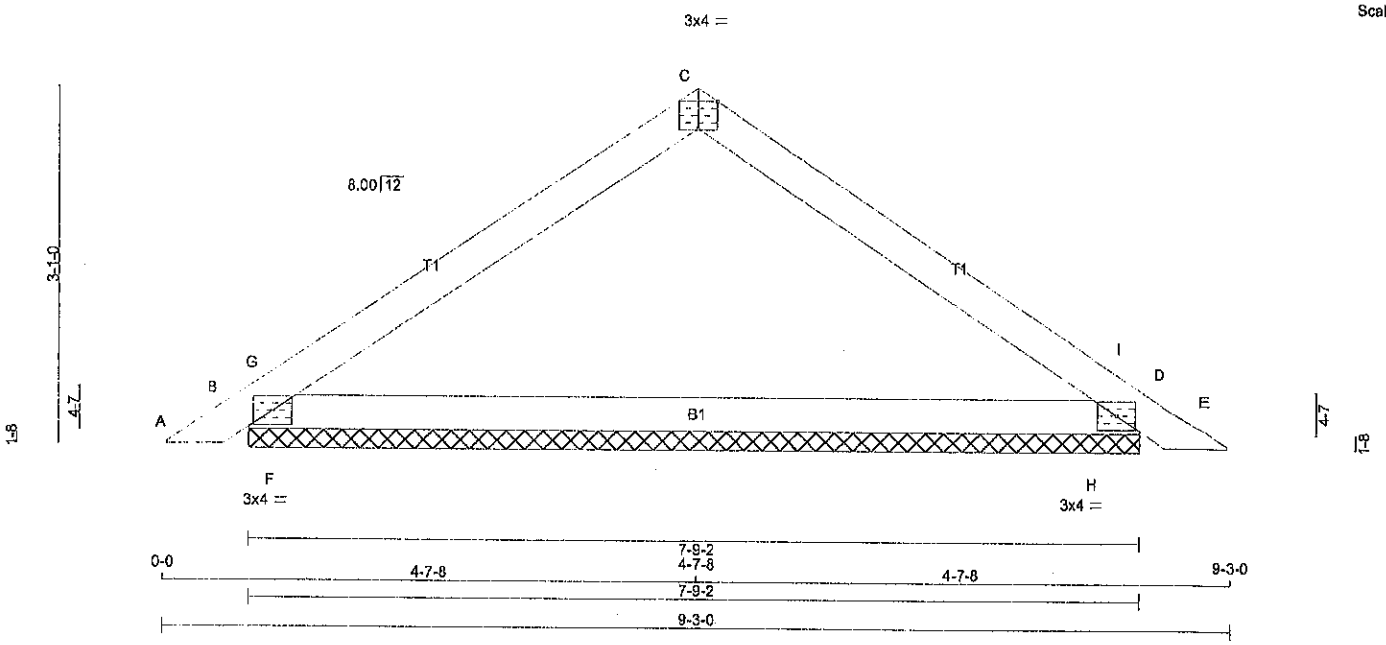

TOTAL WEIGHT = 9 X 21 = 193 lb (M)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TT-p	MT20	3.0	4.0	Edge	2.00
D	TMB1-I	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	387	0	387	0	0	7-9-2	1-8
D	387	0	387	0	0	7-9-2	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	271	190 / 0	0 / 0	0 / 0	0 / 0	81 / 0	0 / 0
D	271	190 / 0	0 / 0	0 / 0	0 / 0	81 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 11	-70.5	-70.5	0.02 (1)	10.00	F- G	0 / 235	0.00 (1)
B- G	-393 / 0	-70.5	-70.5	0.14 (4)	6.25	H- I	0 / 235	0.00 (1)
G- C	-246 / 0	-70.5	-70.5	0.20 (1)	6.25			
C- I	-246 / 0	-70.5	-70.5	0.20 (1)	6.25			
I- D	-393 / 0	-70.5	-70.5	0.14 (4)	6.25			
D- E	0 / 11	-70.5	-70.5	0.02 (1)	10.00			
B- F	0 / 212	-17.5	-17.5	0.12 (4)	10.00			
F- H	0 / 212	-17.5	-17.5	0.17 (1)	10.00			
H- D	0 / 212	-17.5	-17.5	0.12 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.20 (C-G:1) , BC=0.17 (F-H:1) , WB=0.00 (F-G:1) , SSI=0.21 (B-G:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.60

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.36 (D) (INPUT = 0.90)
JSI METAL= 0.09 (D) (INPUT = 1.00)

A-1D073817

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253629	H06	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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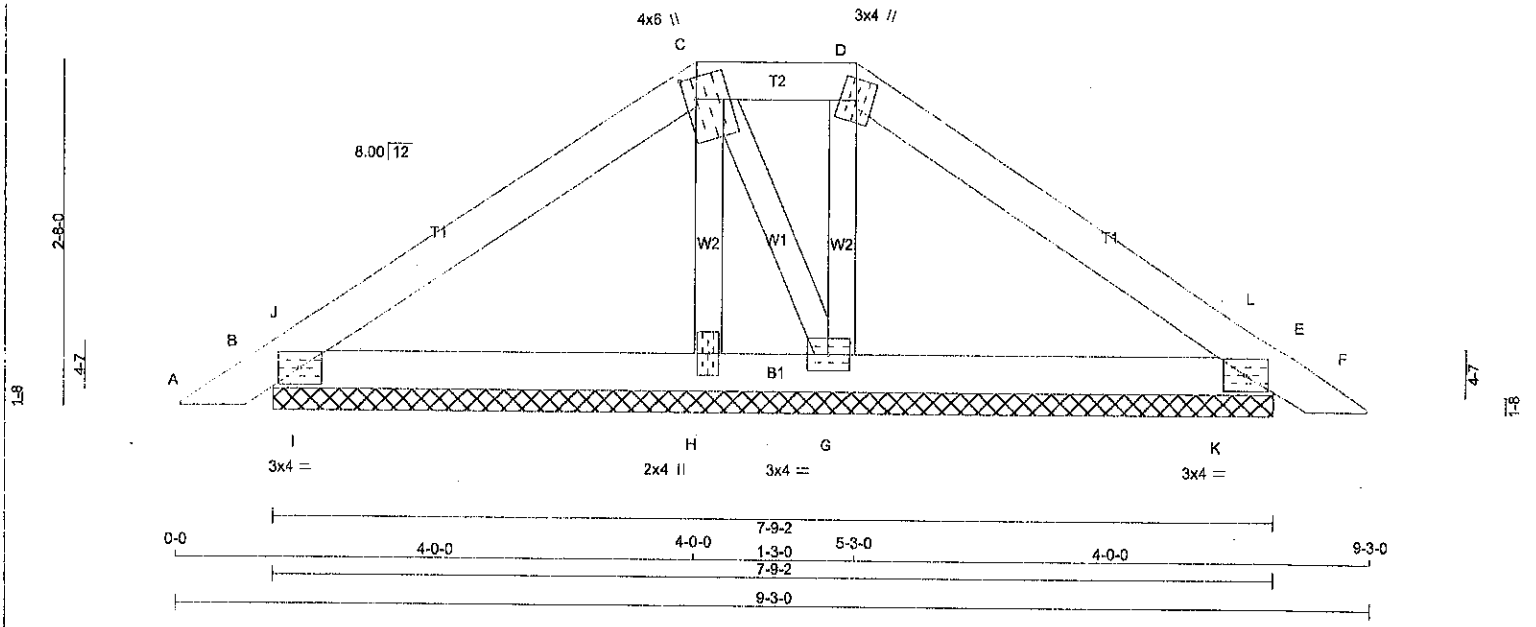

TOTAL WEIGHT = 26 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW+m	MT20	4.0	6.0	2.00	1.00
D	TTW+m	MT20	3.0	4.0		
E	TMB1-I	MT20	3.0	4.0		
G	BMWW1-t	MT20	3.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
B	224	0	224	0	0	7-9-2	1-8
E	214	0	214	0	0	7-9-2	1-8
H	130	0	130	0	0	7-9-2	1-8
G	206	0	206	0	0	7-9-2	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	156	116 / 0	0 / 0	0 / 0	0 / 0	40 / 0	0 / 0
E	148	110 / 0	0 / 0	0 / 0	0 / 0	39 / 0	0 / 0
H	93	54 / 0	0 / 0	0 / 0	0 / 0	39 / 0	0 / 0
G	145	100 / 0	0 / 0	0 / 0	0 / 0	45 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 11	-70.5	-70.5	0.02 (1)	10.00
B- J	-42 / 0	-70.5	-70.5	0.03 (1)	6.25
J- C	-82 / 0	-70.5	-70.5	0.09 (1)	6.25
C- D	-45 / 0	-70.5	-70.5	0.02 (1)	6.25
D- L	-64 / 0	-70.5	-70.5	0.09 (1)	6.25
L- E	-29 / 0	-70.5	-70.5	0.03 (1)	6.25
E- F	0 / 11	-70.5	-70.5	0.02 (1)	10.00
B- I	0 / 65	-17.5	-17.5	0.08 (1)	10.00
I- H	0 / 65	-17.5	-17.5	0.08 (1)	10.00
H- G	0 / 62	-17.5	-17.5	0.06 (1)	10.00
G- K	0 / 50	-17.5	-17.5	0.08 (1)	10.00
K- E	0 / 50	-17.5	-17.5	0.08 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-65 / 0	0.01 (1)
C- G	-37 / 0	0.01 (1)
G- D	-108 / 0	0.02 (1)
I- J	-178 / 11	0.00 (1)
K- L	-180 / 10	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.09 (D-L:1) , BC=0.08 (B-I:1) , WB=0.02 (D-G:1) , SSI=0.14 (E-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)

A-1073816

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253629	H04A	1	2	TRUSS DESC.	

Alpa Roof Truss, Maple

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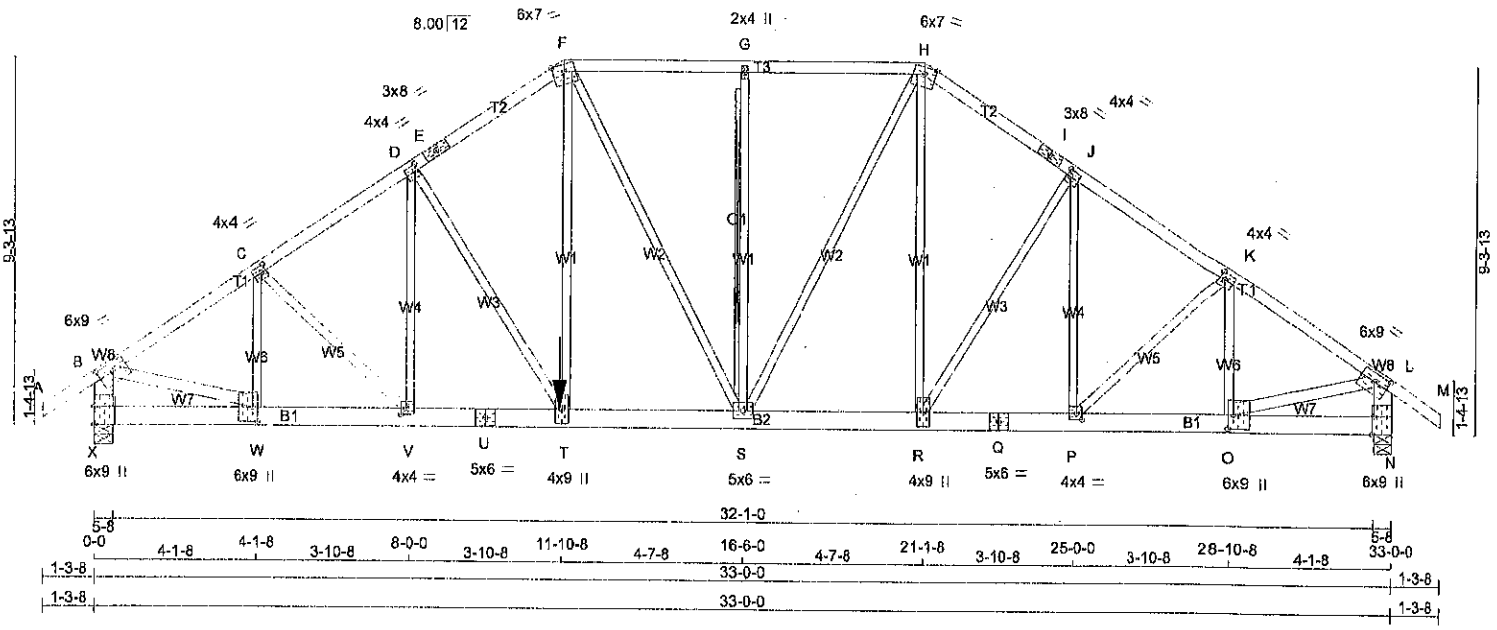


TOTAL WEIGHT = 2 X 189 = 379 lb
[M]

LUMBER
N. L. G. A. RULES

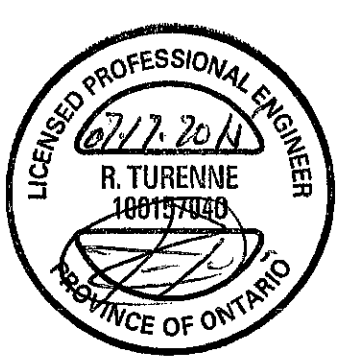
CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - I	2x4	DRY	No.2	SPF
I - M	2x4	DRY	No.2	SPF
X - B	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
X - U	2x6	DRY	No.2	SPF
U - Q	2x6	DRY	No.2	SPF
Q - N	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
B - W	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)		LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- E	1	12	TOP
E- F	1	12	TOP
F- H	1	12	TOP
H- I	1	12	TOP
I- M	1	12	TOP
X- B	2	12	TOP
N- L	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
X- U	2	12	SIDE(162.2)
U- Q	2	12	SIDE(222.8)
Q- N	2	12	SIDE(222.8)
WEBS : (0.122"X3") SPIRAL NAILS			
T- F	1	6	SIDE(178.5)
2x3	1	6	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
X	8685	0	8685	0	0	5-8	5-8
N	9196	0	9196	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
X	6106	4129 / 0	0 / 0	0 / 0	0 / 0	1976 / 0	0 / 0
N	6480	4383 / 0	0 / 0	0 / 0	0 / 0	2097 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X, N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.62FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

2x4 DRY SPF No.2 T-BRACE REQUIRED AT G-S
FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 27	-70.5	-70.5	0.05 (1)	10.00
B- C	-10319 / 0	-70.5	-70.5	0.56 (1)	2.72
C- D	-10564 / 0	-70.5	-70.5	0.51 (1)	2.72
D- E	-9854 / 0	-70.5	-70.5	0.49 (1)	2.83
E- F	-8854 / 0	-70.5	-70.5	0.49 (1)	2.83
F- G	-8776 / 0	-78.0	-78.0	0.44 (1)	3.03
G- H	-8776 / 0	-78.0	-78.0	0.44 (1)	3.03
H- I	-9562 / 0	-70.5	-70.5	0.46 (1)	2.90
I- J	-9562 / 0	-70.5	-70.5	0.46 (1)	2.90
J- K	-10655 / 0	-70.5	-70.5	0.52 (1)	2.70
K- L	-10746 / 0	-70.5	-70.5	0.61 (1)	2.62
L- M	0 / 27	-70.5	-70.5	0.05 (1)	10.00
X- B	-8034 / 0	0.0	0.0	0.29 (1)	5.31
N- L	-8367 / 0	0.0	0.0	0.30 (1)	5.21
X- W	0 / 0	-341.8	-341.8	0.21 (1)	10.00
W- V	0 / 8592	-341.8	-341.8	0.89 (1)	10.00
V- U	0 / 8792	-341.8	-341.8	0.76 (1)	10.00
U- T	0 / 8792	-341.8	-341.8	0.76 (1)	10.00
T- S	0 / 8226	-463.2	-463.2	0.87 (1)	10.00
S- R	0 / 7974	-463.2	-463.2	0.85 (1)	10.00
R- Q	0 / 8869	-463.2	-463.2	0.82 (1)	10.00
Q- P	0 / 8869	-463.2	-463.2	0.82 (1)	10.00
P- O	0 / 8946	-463.2	-463.2	0.76 (1)	10.00
O- N	0 / 0	-463.2	-463.2	0.28 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
W- C	-617 / 0	0.08 (1)
C- V	0 / 278	0.03 (1)
V- D	0 / 1063	0.13 (1)
D- T	-1155 / 0	0.63 (1)
T- F	0 / 4125	0.51 (1)
F- S	0 / 1198	0.15 (1)
S- G	-374 / 0	0.16 (1)
S- H	0 / 1744	0.22 (1)
R- H	0 / 3479	0.43 (1)
R- J	-1758 / 0	0.96 (1)
P- J	0 / 1782	0.22 (1)
P- K	-107 / 0	0.03 (1)
O- K	-158 / 6	0.02 (1)
B- W	0 / 8639	0.76 (1)
O- L	0 / 9203	0.81 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
T	11-9-12	-1442	-1442	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	21.0	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.34")

CSI: TC=0.61 (K-L:1) , BC=0.87 (S-T:1) , WB=0.96 (J-R:1) , SSI=0.38 (S-T:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

A-N073815

CONTINUED ON PAGE 2

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	6.0	9.0	2.50	4.50
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TMWW-t	MT20	4.0	4.0	2.00	1.00
E	TS-t	MT20	3.0	8.0		
F	TTWW-m	MT20	6.0	7.0	Edge	
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	6.0	7.0	Edge	
I	TS-t	MT20	3.0	8.0		
J	TMWW-t	MT20	4.0	4.0	2.00	1.00
K	TMWW-t	MT20	4.0	4.0	2.00	1.50
L	TMVW-t	MT20	6.0	9.0	2.50	4.50
N	BMV1+t	MT20	6.0	9.0	Edge	0.50
O	BMWW+t	MT20	6.0	9.0	4.50	1.50
P	BMWW-t	MT20	4.0	4.0	2.00	1.50
Q	BS-t	MT20	5.0	6.0		
R	BMWW+t	MT20	4.0	9.0		
S	BMWWW-t	MT20	5.0	6.0		
T	BMWW+t	MT20	4.0	9.0		
U	BS-t	MT20	5.0	6.0		
V	BMWW-t	MT20	4.0	4.0	2.00	1.50
W	BMWW+t	MT20	6.0	9.0	4.50	1.50
X	BMV1+t	MT20	6.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1442.0 lbs FACTORED DOWN AT 11-9-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.97 (W) (INPUT = 1.00)

A-1507385(2)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253629	H04	10	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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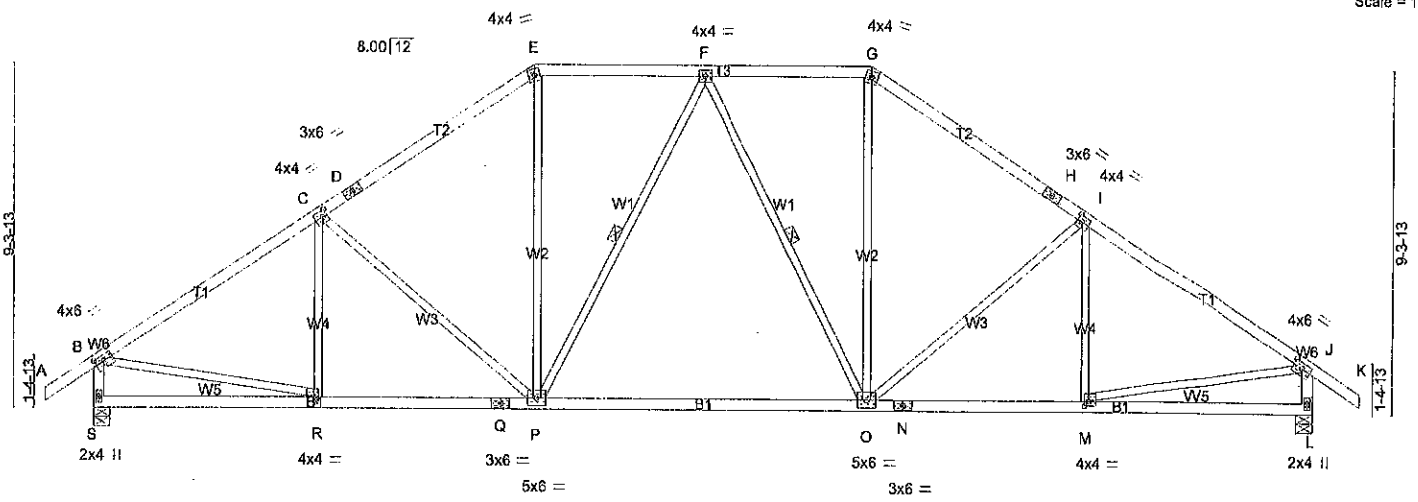

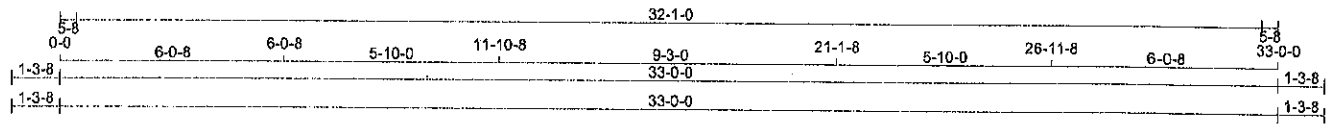

TOTAL WEIGHT = 10 X 147 = 1465 lb

[M][F]

LUMBER

N. L., G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	1.75	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTW-m	MT20	4.0	4.0		
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	4.0	4.0	2.00	1.50
J	TMVW-t	MT20	4.0	6.0	1.75	3.00
L	BMV1+p	MT20	2.0	4.0		
M	BMWW-t	MT20	4.0	4.0	1.50	1.50
N	BS-t	MT20	3.0	6.0		
O	BMWWW-t	MT20	5.0	6.0		
P	BMWWW-t	MT20	5.0	6.0		
Q	BS-t	MT20	3.0	6.0		
R	BMWW-t	MT20	4.0	4.0	1.50	1.50
S	BMV1+p	MT20	2.0	4.0		

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	1583	0	1583	0	0	5-8	1-11
L	1583	0	1583	0	0	5-8	1-11

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1116	750 / 0	0 / 0	0 / 0	0 / 0	366 / 0	0 / 0
L	1116	750 / 0	0 / 0	0 / 0	0 / 0	366 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.62FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF F-P, F-O.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	CSI	LC	MAX UNBRAC LENGTH
A- B	0 / 27	-70.5	-70.5	0.09	(1)	10.00
B- C	-1787 / 0	-70.5	-70.5	0.40	(1)	4.62
C- D	-1566 / 0	-70.5	-70.5	0.37	(1)	4.90
D- E	-1566 / 0	-70.5	-70.5	0.37	(1)	4.90
E- F	-1284 / 0	-78.0	-78.0	0.23	(1)	5.45
F- G	-1284 / 0	-78.0	-78.0	0.23	(1)	5.45
G- H	-1566 / 0	-70.5	-70.5	0.37	(1)	4.90
H- I	-1566 / 0	-70.5	-70.5	0.37	(1)	4.90
I- J	-1787 / 0	-70.5	-70.5	0.40	(1)	4.62
J- K	0 / 27	-70.5	-70.5	0.09	(1)	10.00
S- B	-1536 / 0	0.0	0.0	0.16	(1)	6.64
L- J	-1536 / 0	0.0	0.0	0.16	(1)	6.64
S- R	0 / 0	-17.5	-17.5	0.13	(4)	10.00
R- Q	0 / 1509	-17.5	-17.5	0.37	(1)	10.00
Q- P	0 / 1509	-17.5	-17.5	0.37	(1)	10.00
P- O	0 / 1393	-17.5	-17.5	0.36	(1)	10.00
O- N	0 / 1509	-17.5	-17.5	0.37	(1)	10.00
N- M	0 / 1509	-17.5	-17.5	0.37	(1)	10.00
M- L	0 / 0	-17.5	-17.5	0.13	(4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI	LC
R- C	-168 / 26	0.07	(1)
C- P	-306 / 0	0.39	(1)
P- E	0 / 558	0.13	(1)
P- F	-244 / 0	0.18	(1)
F- O	-244 / 0	0.18	(1)
O- G	0 / 558	0.13	(1)
O- I	-306 / 0	0.39	(1)
M- I	-168 / 26	0.07	(1)
B- R	0 / 1533	0.35	(1)
M- J	0 / 1533	0.35	(1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 8.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.40 (I-J:1) , BC=0.37 (M-O:1) , WB=0.39 (I-O:1) , SSI=0.17 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1666

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (R) (INPUT = 0.90)
JSI METAL= 0.57 (R) (INPUT = 1.00)

A-1073814

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253629	H03	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MiTek Industries, Inc. Fri Jul 17 08:26:59 2015 Page 1
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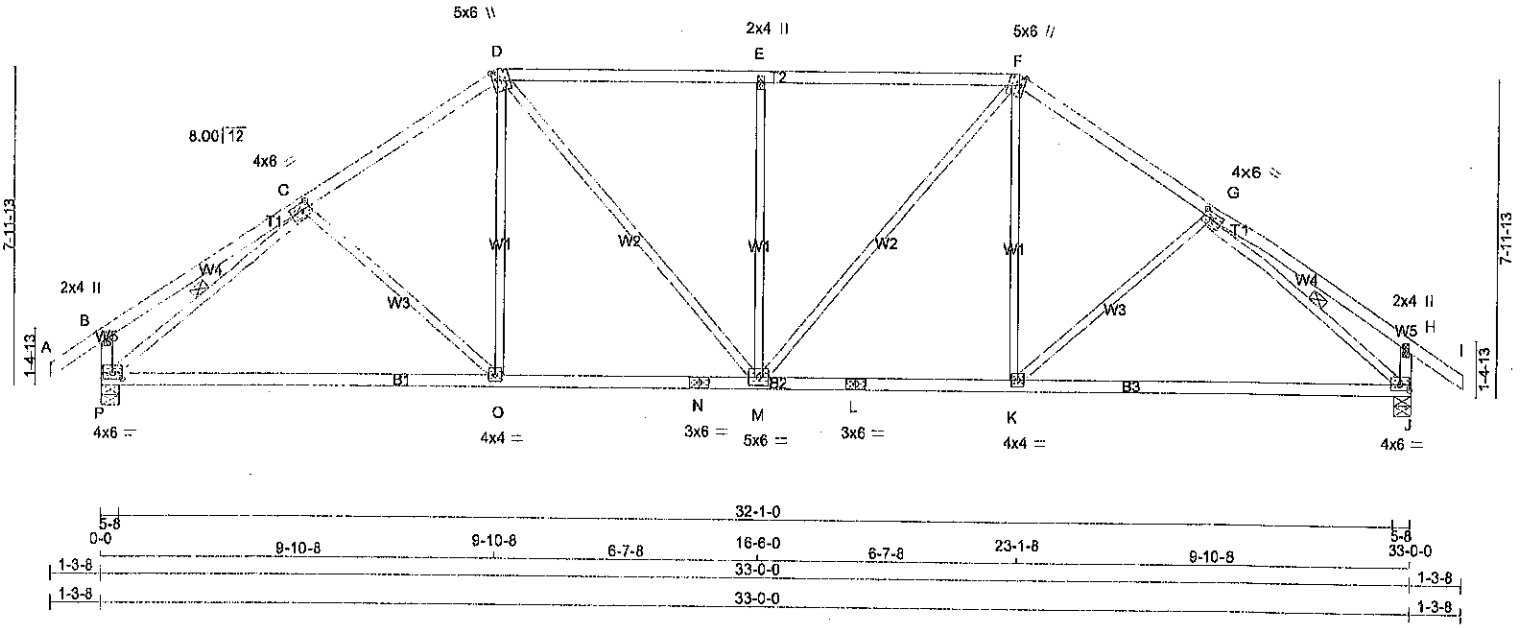


TOTAL WEIGHT = 140 lb
(M)(F)

LUMBER
N. L. G. A. RULES

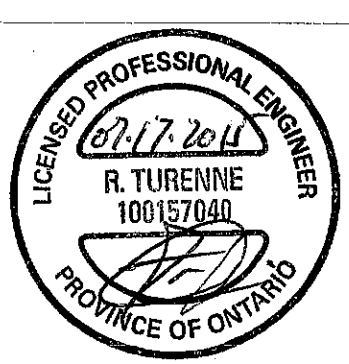
CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWW-t	MT20	4.0	6.0	2.00	2.75
D	TTWW+m	MT20	5.0	6.0	2.50	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.50	1.50
G	TMWW-t	MT20	4.0	6.0	2.00	2.75
H	TMV+p	MT20	2.0	4.0		
J	BMVW1-t	MT20	4.0	6.0	1.75	3.00
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	5.0	6.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMVW1-t	MT20	4.0	6.0	1.75	3.00

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	1548	0	1548	0	0	5-8	1-11
J	1548	0	1548	0	0	5-8	1-11

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1088	750 / 0	0 / 0	0 / 0	0 / 0	338 / 0	0 / 0
J	1088	750 / 0	0 / 0	0 / 0	0 / 0	338 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.72FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF C-P, G-J.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF) CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 27	-70.5	-70.5	0.09 (1)	10.00	C- O	-106 / 34	0.09 (1)
B- C	0 / 22	-70.5	-70.5	0.27 (1)	10.00	O- D	0 / 253	0.06 (4)
C- D	-1618 / 0	-70.5	-70.5	0.25 (1)	4.97	D- M	0 / 423	0.10 (1)
D- E	-1606 / 0	-70.5	-70.5	0.45 (1)	4.72	M- E	-573 / 0	0.70 (1)
E- F	-1606 / 0	-70.5	-70.5	0.45 (1)	4.72	M- F	0 / 423	0.10 (1)
F- G	-1618 / 0	-70.5	-70.5	0.25 (1)	4.97	K- F	0 / 253	0.06 (4)
G- H	0 / 22	-70.5	-70.5	0.27 (1)	10.00	K- G	-106 / 34	0.09 (1)
H- I	0 / 27	-70.5	-70.5	0.09 (1)	10.00	P- C	-1879 / 0	0.55 (1)
P- B	-231 / 0	0.0	0.0	0.02 (1)	7.81	G- J	-1879 / 0	0.55 (1)
J- H	-231 / 0	0.0	0.0	0.02 (1)	7.81			
P- O	0 / 1407	-17.5	-17.5	0.49 (4)	10.00			
O- N	0 / 1330	-17.5	-17.5	0.49 (4)	10.00			
N- M	0 / 1330	-17.5	-17.5	0.49 (4)	10.00			
M- L	0 / 1330	-17.5	-17.5	0.49 (4)	10.00			
L- K	0 / 1330	-17.5	-17.5	0.49 (4)	10.00			
K- J	0 / 1407	-17.5	-17.5	0.49 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.30")

CSI: TC=0.45 (D-E:1) , BC=0.49 (K-M:4) , WB=0.70 (E-M:1) , SSI=0.23 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.60

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(ORY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.46 (G) (INPUT = 1.00)

A-N073813

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253629	H02	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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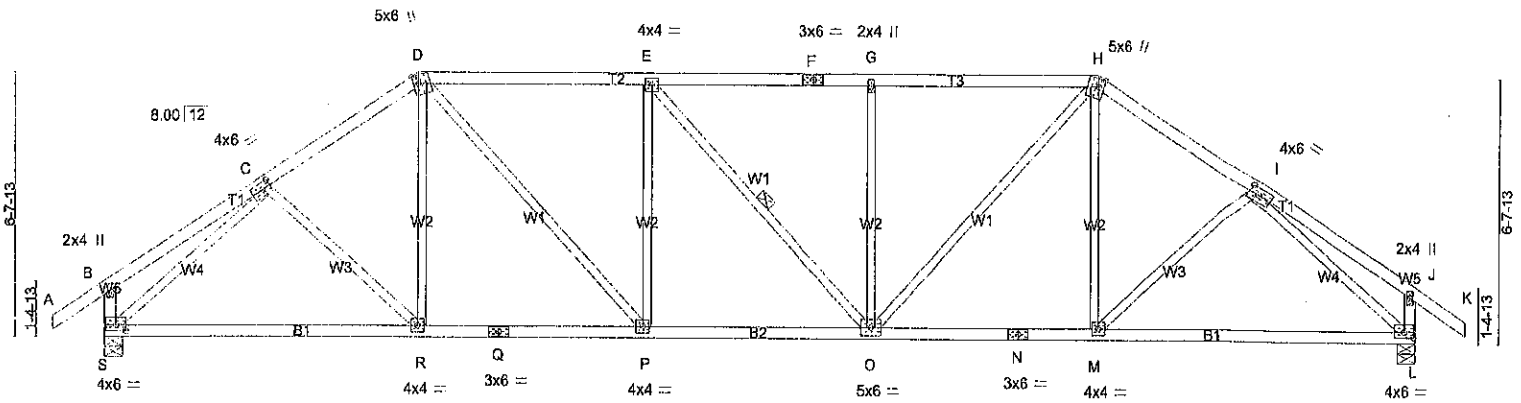

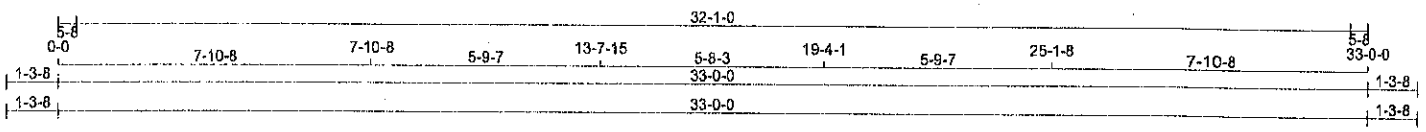

TOTAL WEIGHT = 141 lb
[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWW-t	MT20	4.0	6.0	2.00	2.50
D	TTWW+m	MT20	5.0	6.0	2.50	1.50
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW+m	MT20	5.0	6.0	2.50	1.50
I	TMWW-t	MT20	4.0	6.0	2.00	2.50
J	TMV+p	MT20	2.0	4.0		
L	BMVW1-t	MT20	4.0	6.0	1.75	3.00
M, P, R						
M	BMWW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	6.0		
O	BMWWW-t	MT20	5.0	6.0		
Q	BS-t	MT20	3.0	6.0		
S	BMVW1-t	MT20	4.0	6.0	1.75	3.00

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
S	1648	0	1548	0	0	5-8	1-11
L	1648	0	1548	0	0	5-8	1-11

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1088	750 / 0	0 / 0	0 / 0	0 / 0	338 / 0	0 / 0
L	1088	750 / 0	0 / 0	0 / 0	0 / 0	338 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.50FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/ 2 LENGTH OF E-O.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 27	-70.5	-70.5	0.09 (1)	10.00	C- R	0 / 62	0.02 (4)
B- C	0 / 16	-70.5	-70.5	0.16 (1)	10.00	R- D	0 / 124	0.04 (4)
C- D	-1684 / 0	-70.5	-70.5	0.21 (1)	4.92	D- P	0 / 712	0.16 (1)
D- E	-1867 / 0	-70.5	-70.5	0.41 (1)	4.50	P- E	-437 / 0	0.32 (1)
E- F	-1866 / 0	-70.5	-70.5	0.40 (1)	4.50	E- O	-2 / 0	0.00 (1)
F- G	-1866 / 0	-70.5	-70.5	0.40 (1)	4.50	O- G	-437 / 0	0.32 (1)
G- H	-1866 / 0	-70.5	-70.5	0.40 (1)	4.51	O- H	0 / 710	0.16 (1)
H- I	-1685 / 0	-70.5	-70.5	0.21 (1)	4.92	M- H	0 / 124	0.04 (4)
I- J	0 / 16	-70.5	-70.5	0.16 (1)	10.00	M- I	0 / 62	0.02 (4)
J- K	0 / 27	-70.5	-70.5	0.09 (1)	10.00	S- C	-1874 / 0	0.89 (1)
S- B	-206 / 0	0.0	0.0	0.02 (1)	7.81	I- L	-1874 / 0	0.89 (1)
L- J	-206 / 0	0.0	0.0	0.02 (1)	7.81			
S- R	0 / 1364	-17.5	-17.5	0.35 (1)	10.00			
R- Q	0 / 1388	-17.5	-17.5	0.35 (1)	10.00			
Q- P	0 / 1388	-17.5	-17.5	0.35 (1)	10.00			
P- O	0 / 1867	-17.5	-17.5	0.36 (1)	10.00			
O- N	0 / 1388	-17.5	-17.5	0.36 (1)	10.00			
N- M	0 / 1388	-17.5	-17.5	0.36 (1)	10.00			
M- L	0 / 1364	-17.5	-17.5	0.35 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.41 (D-E:1) , BC=0.36 (O-P:1) , WB=0.89 (H-L:1) , SSI=0.19 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.60

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (I) (INPUT = 0.90)
JSI METAL= 0.46 (I) (INPUT = 1.00)

A-150738 12

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253629	H01	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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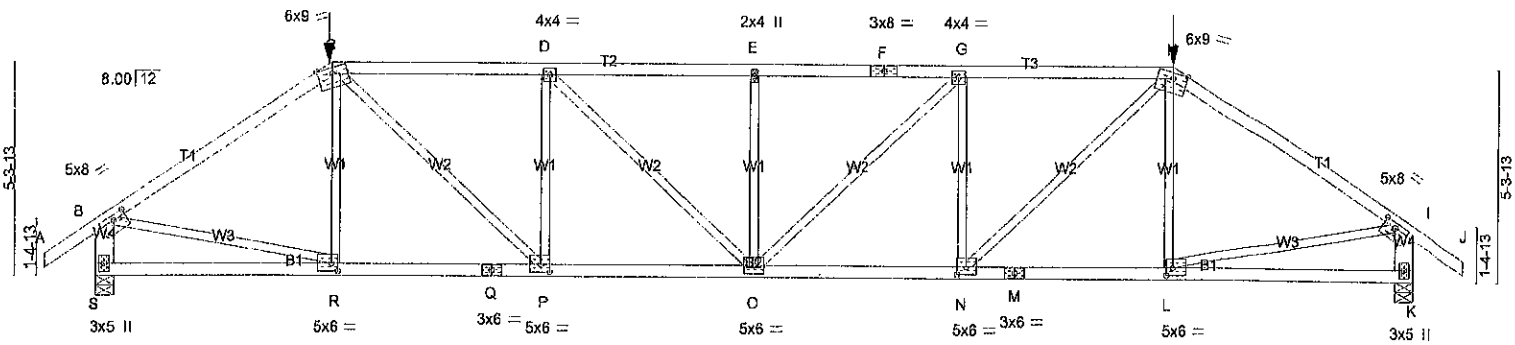
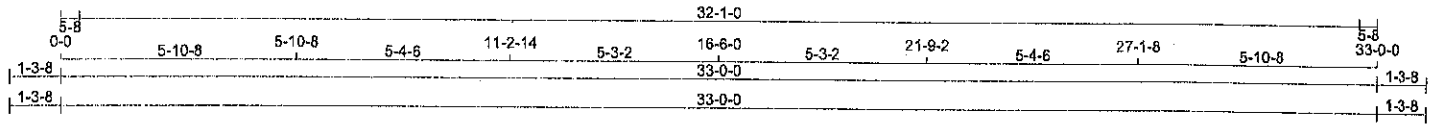




TOTAL WEIGHT = 137 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	1650F 1.5E	SPF
F - H	2x4	DRY	1650F 1.5E	SPF
H - J	2x4	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	1.50	3.75
C	TTWW-m	MT20	6.0	9.0	Edge	4.25
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW-m	MT20	6.0	9.0	Edge	4.25
I	TMVW-t	MT20	5.0	8.0	1.50	3.75
K	BMV1+p	MT20	3.0	5.0		
L	BMWW-t	MT20	5.0	6.0	2.25	2.00
M	BS-t	MT20	3.0	6.0		
N	BMWW-t	MT20	5.0	6.0	2.25	2.75
O	BMWWW-t	MT20	5.0	6.0		
P	BMWW-t	MT20	5.0	6.0	2.25	2.75
Q	BS-t	MT20	3.0	6.0		
R	BMWW-t	MT20	5.0	6.0	2.25	2.00
S	BMV1+p	MT20	3.0	5.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	2855	0	2855	0	0	5-8	3-12
K	2855	0	2855	0	0	5-8	3-12

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	2011	1362 / 0	0 / 0	0 / 0	0 / 0	649 / 0	0 / 0
K	2011	1362 / 0	0 / 0	0 / 0	0 / 0	649 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.89FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 27	-70.5	-70.5	0.10 (1)	10.00		R- C	-301 / 85	0.13 (1)
B- C	-3492 / 0	-70.5	-70.5	0.99 (1)	2.94		C- P	0 / 2087	0.52 (1)
C- D	-4421 / 0	-138.7	-138.7	0.77 (1)	3.08		P- D	-1256 / 0	0.55 (1)
D- E	-4870 / 0	-138.7	-138.7	0.81 (1)	2.89		D- O	0 / 620	0.15 (1)
E- F	-4870 / 0	-138.7	-138.7	0.81 (1)	2.89		O- E	-873 / 0	0.29 (1)
F- G	-4870 / 0	-138.7	-138.7	0.81 (1)	2.89		O- G	0 / 620	0.15 (1)
G- H	-4421 / 0	-138.7	-138.7	0.77 (1)	3.08		N- G	-1256 / 0	0.55 (1)
H- I	-3492 / 0	-70.5	-70.5	0.99 (1)	2.94		N- H	0 / 2087	0.52 (1)
I- J	0 / 27	-70.5	-70.5	0.10 (1)	10.00		L- H	-301 / 85	0.13 (1)
S- B	-2770 / 0	0.0	0.0	0.20 (1)	6.24		B- R	0 / 2949	0.73 (1)
K- I	-2770 / 0	0.0	0.0	0.20 (1)	6.24		L- I	0 / 2949	0.73 (1)
S- R	0 / 0	-34.5	-34.5	0.30 (4)	10.00				
R- Q	0 / 2896	-34.5	-34.5	0.68 (1)	10.00				
Q- P	0 / 2896	-34.5	-34.5	0.68 (1)	10.00				
P- O	0 / 4421	-34.5	-34.5	0.88 (1)	10.00				
O- N	0 / 4421	-34.5	-34.5	0.88 (1)	10.00				
N- M	0 / 2896	-34.5	-34.5	0.68 (1)	10.00				
M- L	0 / 2896	-34.5	-34.5	0.68 (1)	10.00				
L- K	0 / 0	-34.5	-34.5	0.30 (4)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE
C	5-10-8	-302	-302	---	FRONT	VERT	TOTAL
H	27-1-8	-302	-302	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 5-10-8
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.25")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 875 (0.45")

CSI: TC=0.99 (H-I:1) , BC=0.68 (N-O:1) , WB=0.73 (I-L:1) , SSI=0.38 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.77 (Q) (INPUT = 1.00)

A-H073811

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253629	H01	1	1	TRUSS DESC.	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 301.8 lbs FACTORED DOWN AT 27-1-8, AND 301.8 lbs FACTORED DOWN AT 5-10-8 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

A-1073811 (2)

LUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

MATERIAL: 18 gauge

FINISH: G90 galvanized

DESIGN:

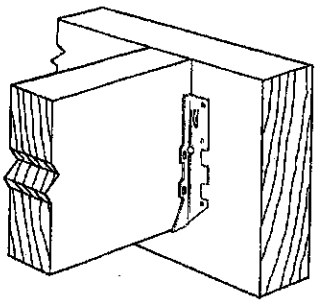
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

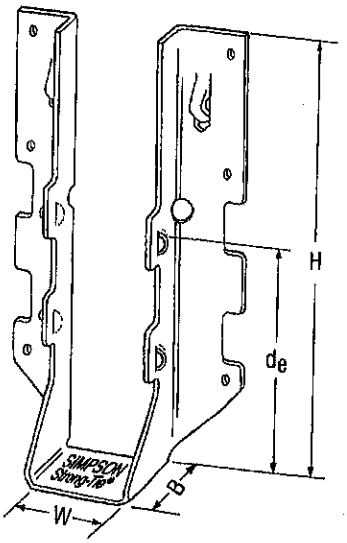
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

- These hangers cannot be modified.



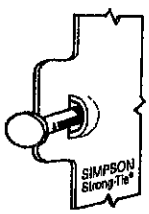
Typical LUS Installation



LUS28

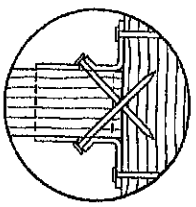
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LUS24	18	1 9/16	3 1/8	1 3/4	1 15/16	4-10d	2-10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 13/16	4-16d	2-16d	835	2020	590	1435
LUS26	18	1 9/16	4 3/4	1 3/4	3 5/8	4-10d	4-10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 7/8	2	4	4-16d	4-16d	1720	2595	1545	1920
LUS26-3	18	4 5/8	4 9/16	2	3 1/4	4-16d	4-16d	1720	2595	1545	2340
LUS28	18	1 9/16	6 5/8	1 3/4	3 3/4	6-10d	4-10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	6-16d	4-16d	1720	3325	1545	2575
LUS28-3	18	4 5/8	6 1/4	2	3 1/4	6-16d	4-16d	1720	3325	1545	2375
LUS210	18	1 9/16	7 13/16	1 3/4	3 7/8	8-10d	4-10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	8-16d	6-16d	2580	4500	2320	3195
LUS210-3	18	4 5/8	8 9/16	2	5 1/4	8-16d	6-16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.

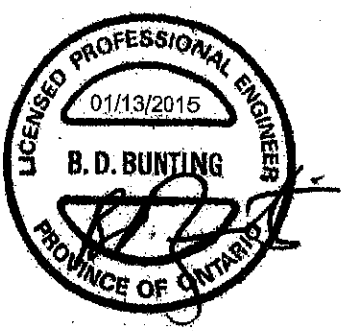


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Top View.



TECHNICAL BULLETIN

HUS/LJS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: See table

FINISH: G90 galvanized

DESIGN:

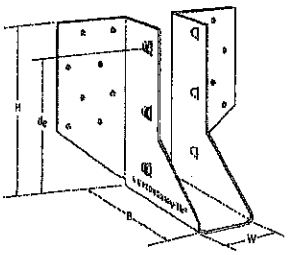
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15% No further increase is permitted
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

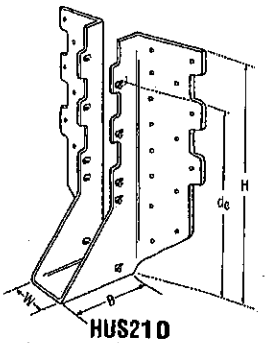
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

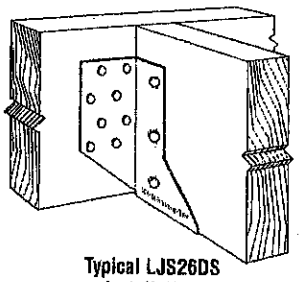
- See current catalogue for options



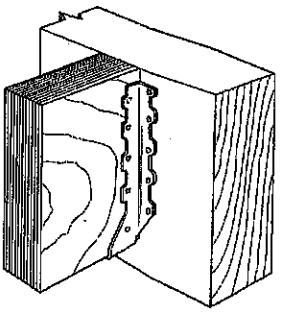
LJS26DS



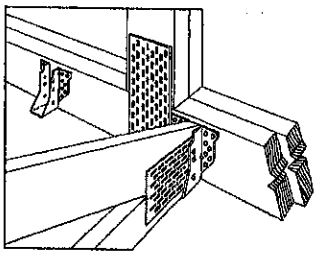
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS Installation



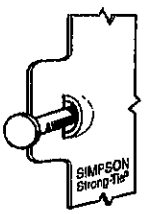
Typical HUS Installation



Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

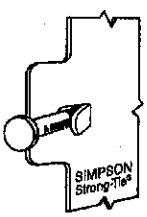
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LJS26DS	18	1 9/16	5	3½	4⅝	16-16d	6-16d	2055	4265	1460	4115
HUS26	16	1⅝	5 3/8	3	3 15/16	14-16d	6-16d	2705	4940	2065	3875
HUS28	16	1⅝	7 9/32	3	6 3/32	22-16d	8-16d	3605	5365	2675	4345
HUS210	16	1⅝	9 3/32	3	7 31/32	30-16d	10-16d	4505	5795	4010	4740
HUS1.81/10	16	1 13/16	9	3	8	30-16d	10-16d	4505	6450	4010	5200

1. d_e is the distance from the seat of the hanger to the highest joist nail.

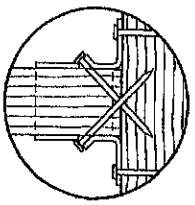


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

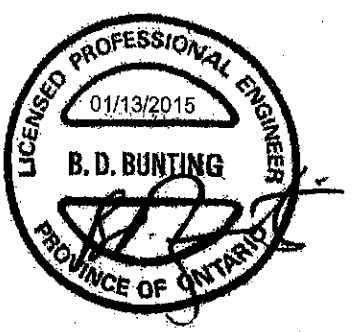
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



TC – Truss Connectors

SIMPSON Strong-Tie®

The TC truss connector is an ideal connector for scissor trusses and can allow horizontal movement up to 1¼". The TC also attaches plated trusses to top plates or sill plates to resist uplift forces. Typically used on one or both ends of truss as determined by the building designer.

MATERIAL: 16 gauge

FINISH: G90 galvanized

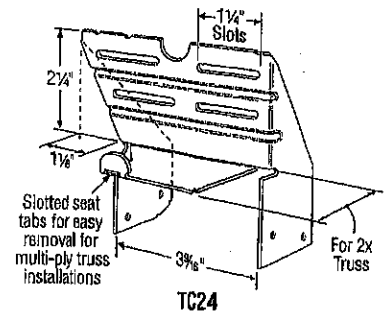
DESIGN: Factored resistances are in accordance with CSA O86-14

INSTALLATION:

- Use all specified fasteners
- Nails: 10d = 0.148" dia. x 3" long common wire, 10d x 1½ = 0.148" dia. x 1½" long.
- Drive 10d nails into the truss at the inside end of the slotted holes (inside end is towards the centre of the truss) and clinch on the back side. Do not seat these nails into the truss–allow room under the nail head for movement of the truss with respect to the wall.

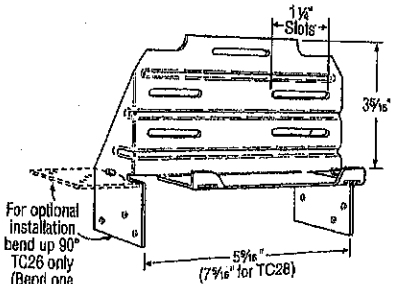
Optional TC Installation

- Bend one flange up 90°. Drive specified nails into the top and face of the top plates or install Titen® screws into the top and face of masonry wall. See optional load tables and installation details.



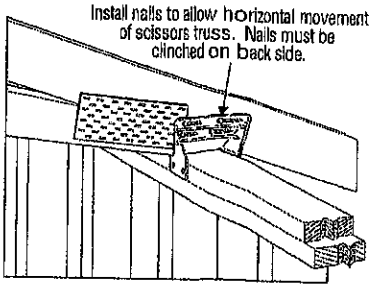
TC24

U.S. Patent 4,932,173

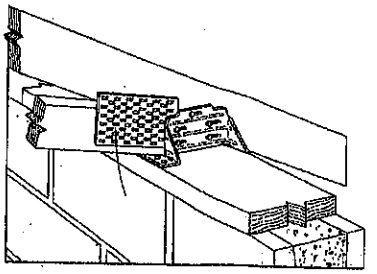


TC26

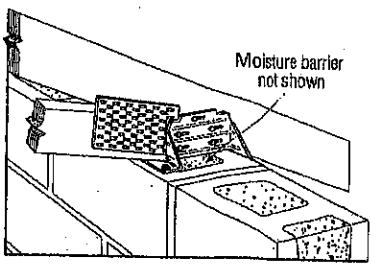
(TC28 Similiar)



Typical TC24 Installation



Optional TC26 Installation for Grouted Concrete Block using a Wood Nailer
(8", 10", 12" Wall Installation Similar)



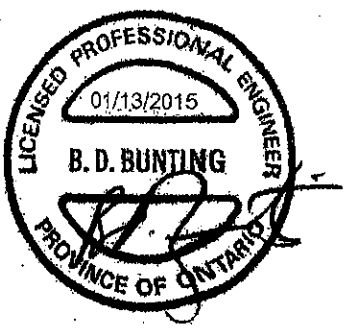
Optional TC26 Installation for Grouted Concrete Block using Titen Screws

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K_D = 1.15)	Uplift (K_D = 1.15)
			lbs	lbs
TC24	4-10d	4-10d	605	430
TC26	5-10d	6-10d	1015	720
TC28	5-10d	6-10d	1015	720

OPTIONAL TC INSTALLATION TABLE

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K_D = 1.15)	Uplift (K = 1.15)
			lbs	lbs
TC26	5-10d	6-10dx1½	810	660
	5-10d	6-10d	930	660

1. Factored resistances have been increased 15% for earthquake or wind loading; no further increase allowed; reduce where other loads govern.
2. Grout strength is 15 MPa minimum.
3. Optional TC26 installation with 10d nails requires minimum 3" top plate thickness.
4. TC26 fastened to grouted concrete block with (6) - ⅜" x 2¼" Titen screws has a factored uplift resistance of 275 lbs.



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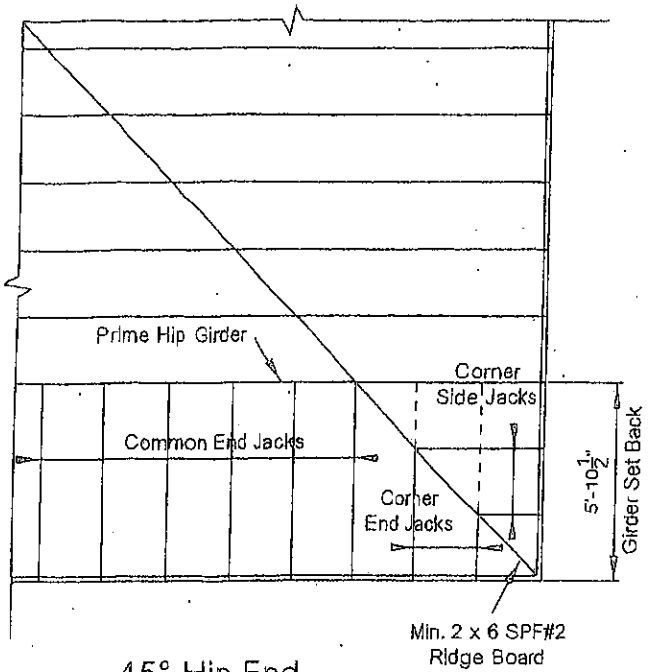


45° Hip End

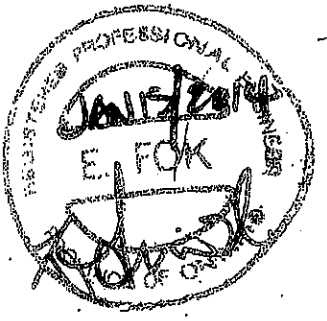
LUMBER SPECIFICATION

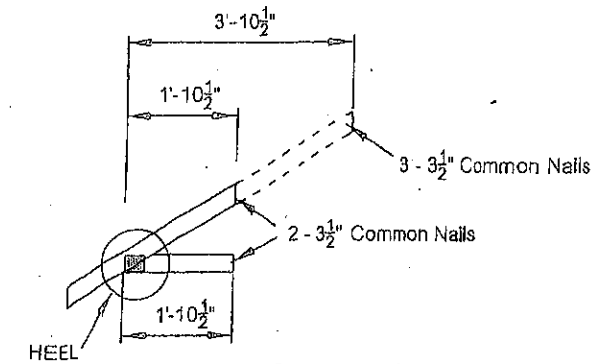
TOP CHORD	:	2 x 4 SPF#2
BOTTOM CHORD	:	2 x 4 SPF#2
WEBS	:	2 x 3 SPF#2

UNLESS OTHERWISE SHOWN

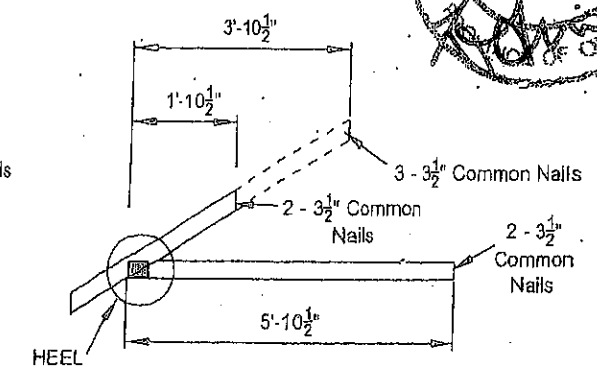
DESIGN LOAD

TOP CHORD SNOW LOAD	:	40.5 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD	:	50.5 P.S.F

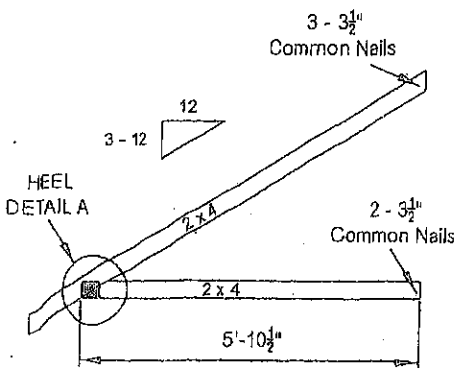




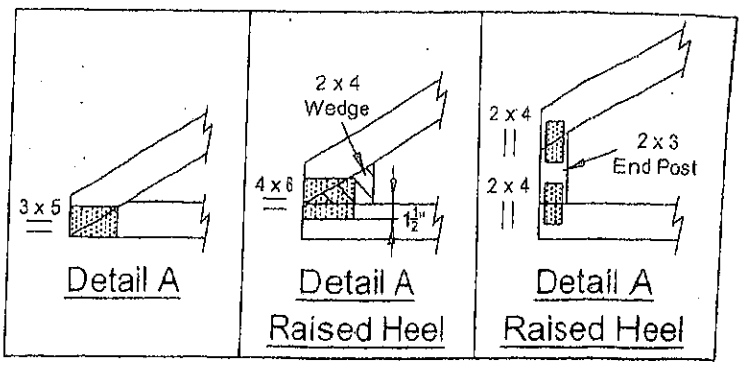
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

CS-51008

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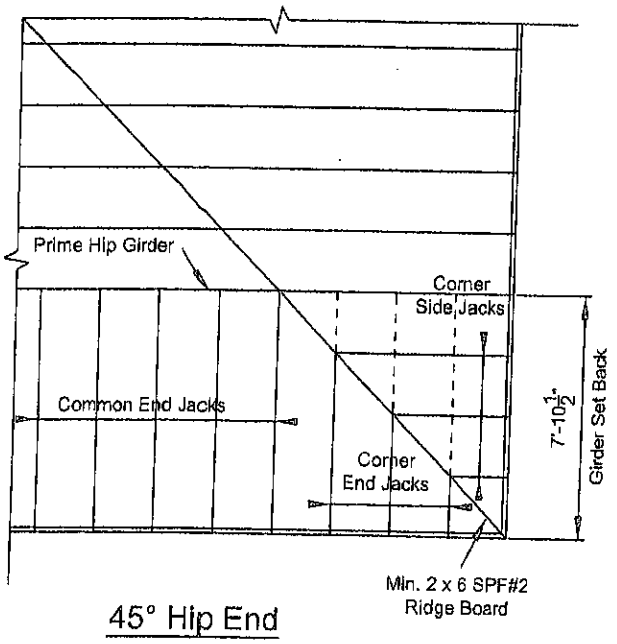


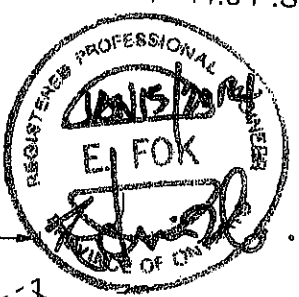
45° Hip End

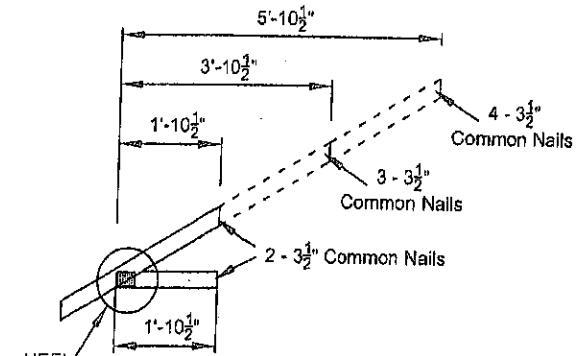
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

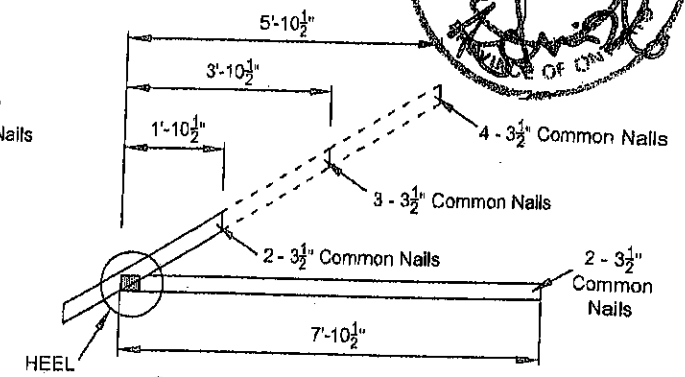
DESIGN LOAD

TOP CHORD SNOW LOAD	:	34.8 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD	:	44.8 P.S.F

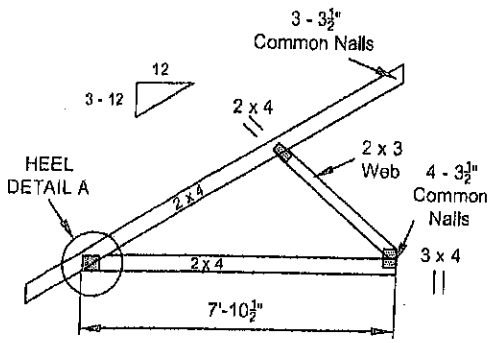




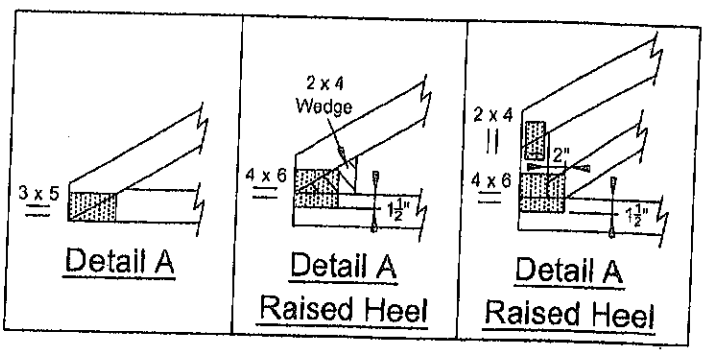
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

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