

Mitek Ver: 7.5.0



CONVENTIONAL
FRAMING BY
OTHERS

Piggyback
 (A-1073781
 A-1073851
 A-1073852
 A-1073873)

HANGERS
// LUS24
X HHUS26-2

12" FIN. OH.
RSD. HEEL - 0" DROP
(2" x 6" FASCI) - FM
ASPHALT SHINGLES
2" X 6" BRG/BRICK
*PIGGYBACK TRS
PURLINS BY OTHERS

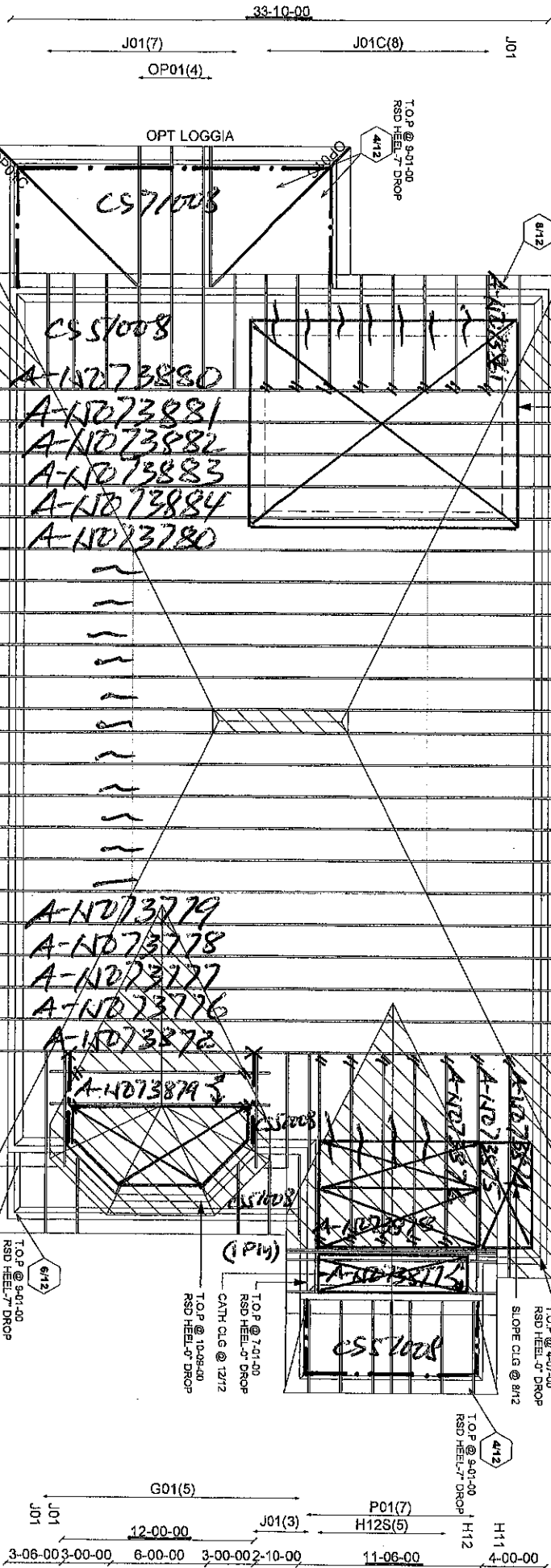
8-00-00

54-02-00

H15
H15

3-00-00	6-01-00	5-00-00
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HANGERS
// LUS24
X HHUS26-2



ALL CONVE FRAMINGS TO CONFORM WITH PART 9 OF O.B.C 2012
ROOF PARTS OF THE CROSS MEET
OVER TRUSSES TO BE 2x4 @ 4' & 2x4 @ C
WITH A VERT POST TO THE TRUSS
UNDERNEATH AT EACH CROSS PT
VERT POST LONGER THAN 6'
TO HAVE LATERAL BRACING SO
THAT THE DIST BETWEEN END PT
& BETWEEN ROWS OF BRACING
DOES NOT EXCEED 6'

8-00-00

60-11-D0

H06(12)

H01T
H02T
H03T
H04T
H05T

OPT TRAY CLG
RAISED - 12"
@ 12x12 LP
INSET 13"
13-0" X 17-0" STD

16x12

H07
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H01

2-04-00 5-00-00
H14
H13
H13
4

李 强

Mitek Ver: 7.5.0

CONVENTIONAL
FRAMING BY
OTHERS

riggyback

12" FIN. OH.
RSD. HEEL - 0" DROP
(2" x 6" FASCIA) - FM
ASPHALT SHINGLES
2" X 6" BRG/BRICK
PIGgyBACK TRS
PURLINS BY OTHERS

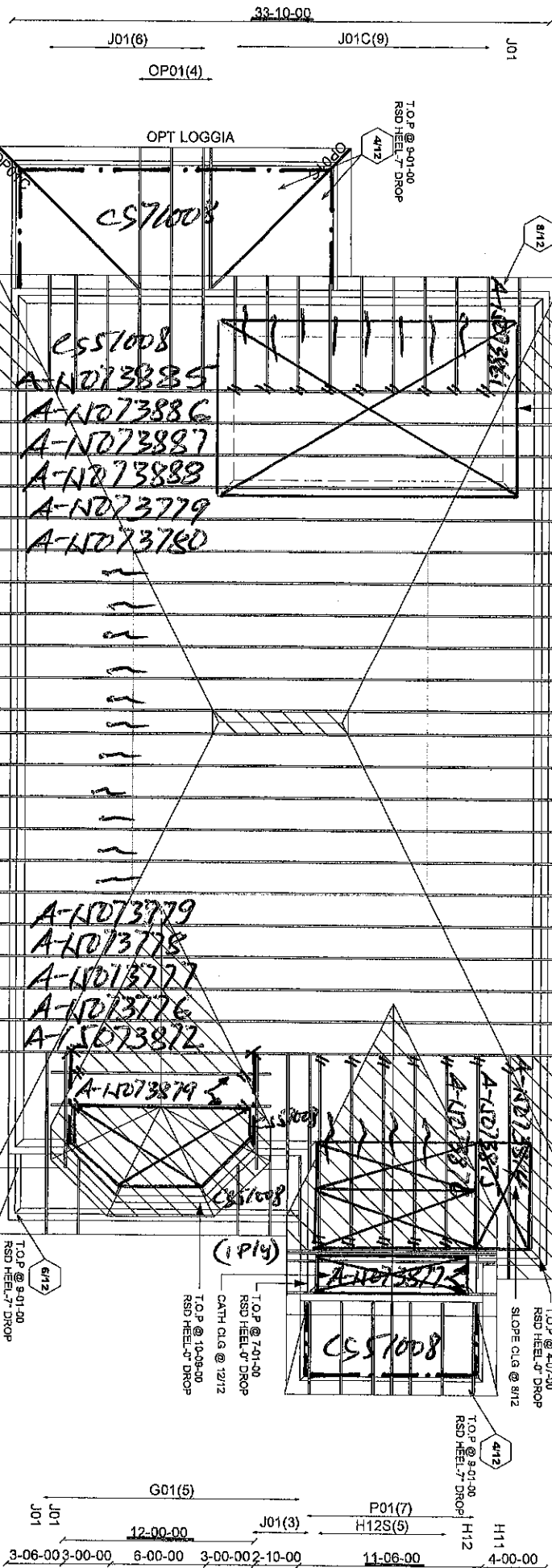
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HANGERS
// LUS24
X HHUS26-2



ALL CONVEY FRAMING TO CONFORM WITH PART 8 OF O.S.C. 2012. ROOF RATTENS THAT CROSS WEEP OVER TRUSSES TO BE 2X4 SPS @ 24" O.C. WITH A VERT POST TO THE TRUSS UNDERBATH AT EACH CROSS PT. VERT POST LONGER THAN 6" TO HAVE LATERAL BRACING SO THAT THE DIST. BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 5'

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H13

REPORT

ALL CONN. FRUSTRATING TO CONFORM WITH PART 9 OF O.B.C. 2012
ROOF RAFTERS THAT CROSS WERT
OVER TRUSSES TO BE 2X4 SPACED 24" O.C.
WITH A VERT. POST TO THE TRUSSES
UNDERNEATH AT EACH CROSS PT.
VERT. POST LONGER THAN 5'
TO HAVE LATERAL BRACING SO
THAT THE DIST. BETWEEN END PT.
& BETWEEN ROWS OF BRACING
DOES NOT EXCEED 8'



HANGERS
// LUS24
/ LJS26DS
X HHUS26-2

 CONVENTIONAL FRAMING BY OTHERS

Mitek Ver. 7.6.2

Job Track: 40297
Layout ID: 258853
Plan Log: 82444

Builder / Location:	GOLD PARK / KLEINBURG	
Project	HUNTINGTON & NASHVILLE	
Date:	9/2/2015	Designer's change

Model / Elevation: **STO & SAT 2ND FLR**
42-5 REV 1 / B
LOT 70 REAR WING R405

THESE DRAWINGS CONSTITUTE THE PROPERTY OF ALPHA ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY ALPHA ROOF TRUSSES INC. AND WILL BE RETRACTED BY ALPHA ROOF TRUSSES INC. IE LITIGATION FOR ANY OTHER PURPOSE



Job Track: 40297
Layout ID: 253645
Plan Log: 79711

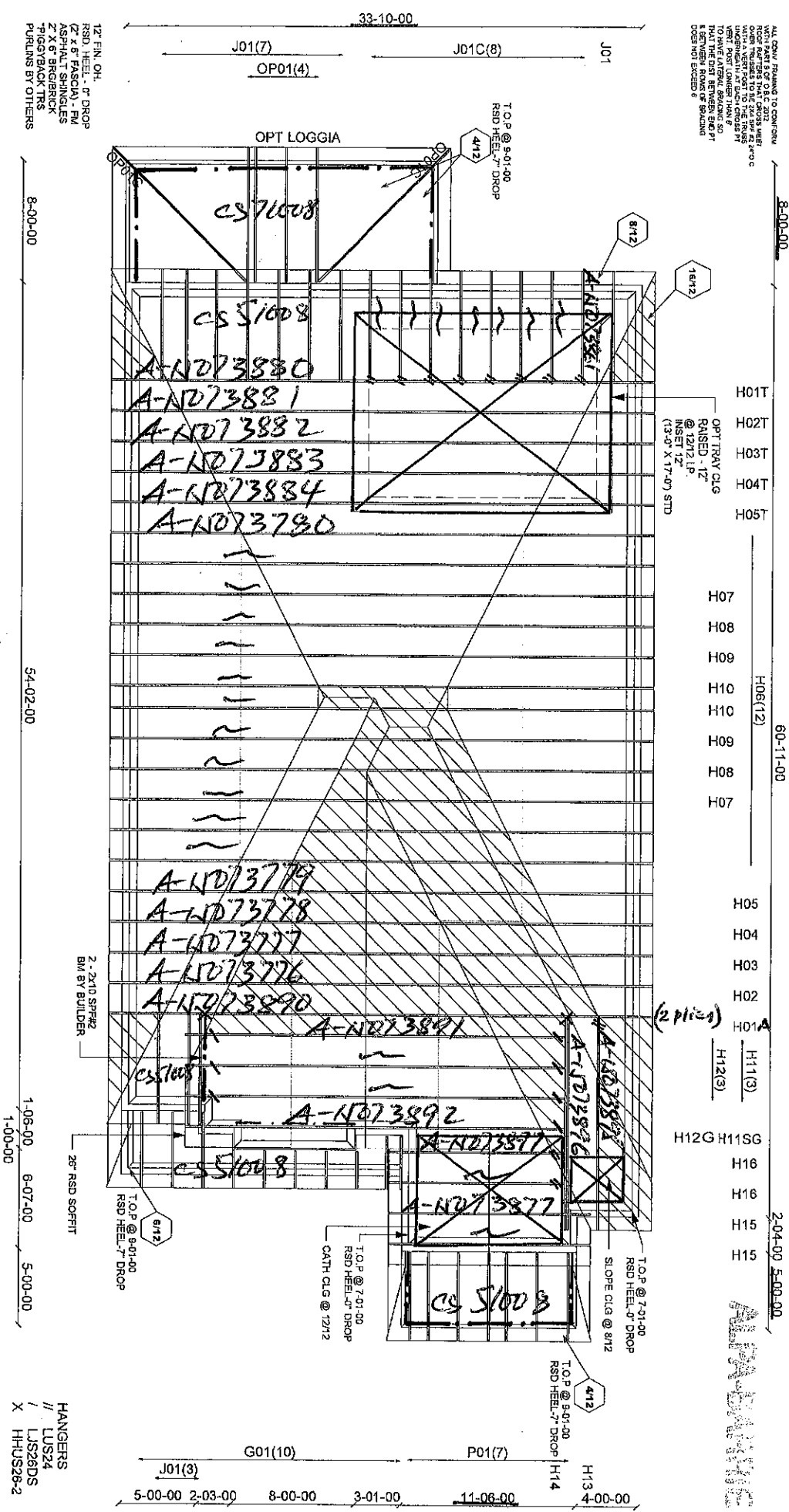
Builder / Location:
GOLD PARK / KLEINBURG
Project: **HUNTINGTON & NASHVILLE**
Date: 4/29/2015 Designer: chryle

Model / Elevation:
42-5 / B OPT TRAY
THESE DRAWINGS CONSTITUTE THE PROPERTY OF ALPHA ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY ALPHA ROOF TRUSSES INC AND WILL BE RETRACTED BY ALPHA ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

CONVENTIONAL
FRAMING BY
OTHERS

Mikek Ver. 7.5.0

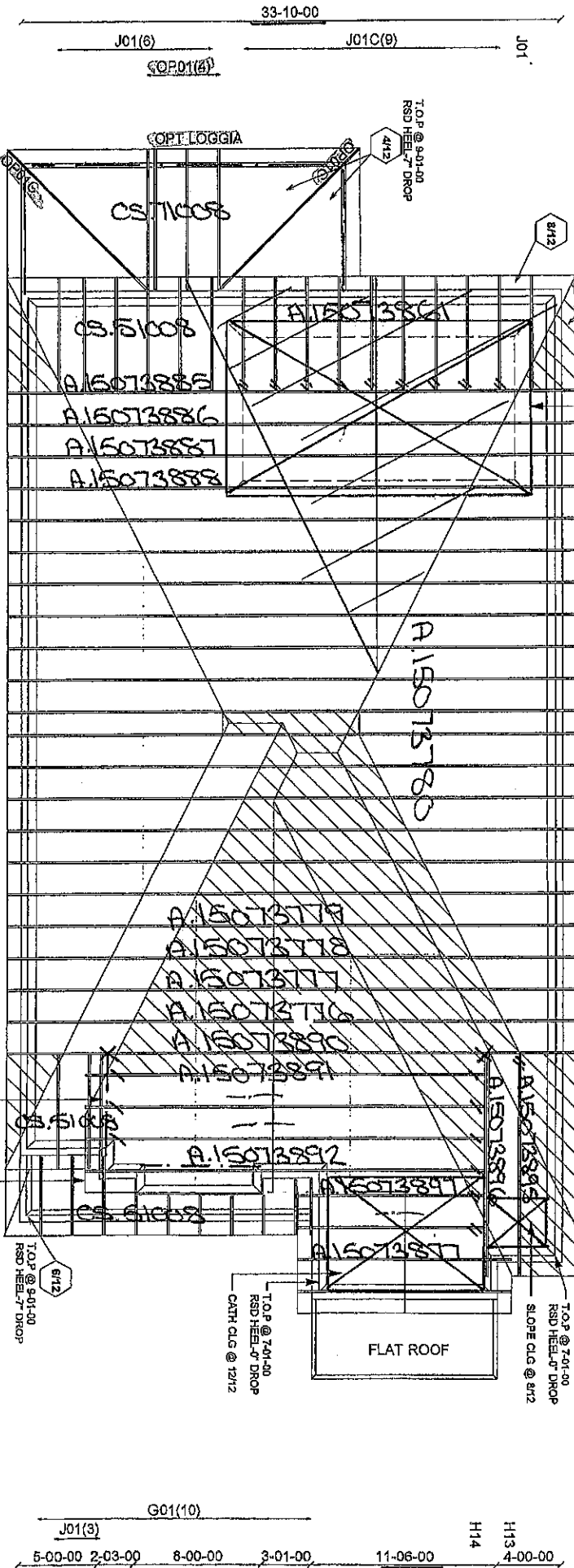
Piggyback
(A-15073781
A-15073851
A-15073852
A-15073873
A-15073893
A-15073894)



ALPHA-BARRIE

ALL DRAWINGS ARE TO BE CONSIDERED AS A GUIDE ONLY. THE CONTRACTOR SHALL BE RESPONSIBLE FOR VERIFYING THE ACCURACY OF ALL DIMENSIONS AND CONDITIONS PRIOR TO CONSTRUCTION. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND APPROVALS FROM THE APPROPRIATE AGENCIES. THE CONTRACTOR SHALL BE RESPONSIBLE FOR MAINTAINING THE SAFETY OF THE WORK AREA AT ALL TIMES. THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROTECTING ALL EXISTING UTILITIES AND STRUCTURES. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE ENVIRONMENT. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE PUBLIC. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE PROPERTY. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE EQUIPMENT. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE MATERIALS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE FINISHES. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE STRUCTURE. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE INTERIORS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE EXTERIORS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE LANDSCAPE. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE ADJACENT PROPERTIES. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE NEIGHBORHOOD. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE COMMUNITY. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE COUNTRY. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE WORLD.

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H15



12\"/>

8-00-00

54-02-00

1-06-00

6-07-00

HANGERS
// LUS24
/ LUS20S
X HUS26-2

A.150738781
A.15073851-2
A.15073873
A.15073893-4
PICKYBACK

CONVENTIONAL
FRAMING BY
OTHERS

PL 85348 (LOT 70)
88062



Job Track 40297
Layout ID: 258855
Plan Log: 82444

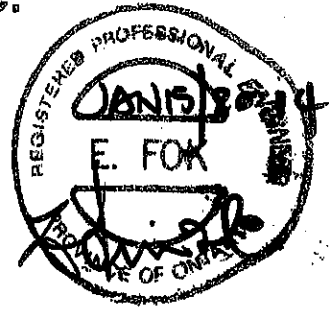
Builder / Location:
GOLD PARK / KLEINBURG
Project: HUNTINGTON & NASHVILLE
Date: 9/2/2015 Designer: cheryle

Model / Elevation:
42-5 REV 1 / B OPT 2ND W/TRAY
KOT 70
REAR VIEW
THESE DRAWINGS CONSTITUTE THE PROPERTY OF ALPHA ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY ALPHA ROOF TRUSSES INC AND WILL BE RETRACTED BY ALPHA ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

Mike Ver 7.5.0

Stracon Engineering Inc.

69 Graydon Crescent
Richmond Hill, Ontario
L4B 3W7
(905) 832-2250 Fax (905) 832-0286



RESPONSIBILITIES

1. Stracon Engineering Inc. is responsible for the design of trusses as individual components.
2. It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities with jurisdictions.
3. All dimensions are to be verified by owner contractor, architect or other authority before manufacture.
4. Stracon Engineering Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Stracon Engineering Inc. drawings is specified for the truss as a component only and forms an integral part of the truss design.
5. It is the truss manufacture's responsibility to ensure that trusses are manufactured in conformance with Stracon Engineering Inc. specifications outlined below.

SPECIFICATIONS

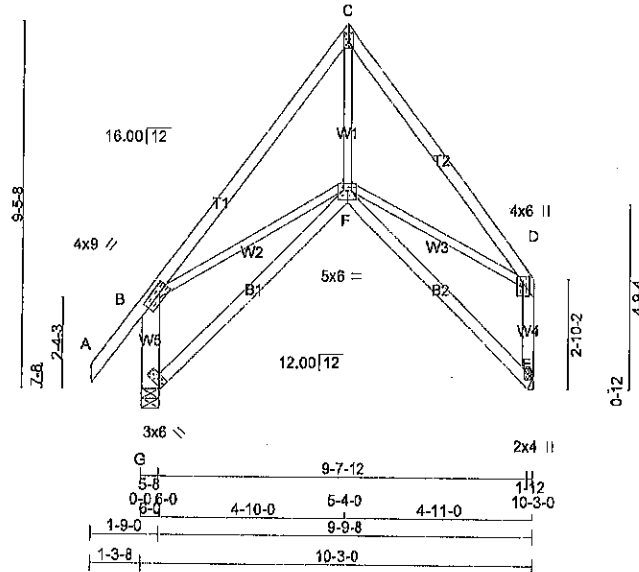
1. Trusses designed by Stracon Engineering Inc. conforms to the relevant section of the Ontario Building Code of Canada (Part 9 or Part 4) or to the Canadian code for farm buildings, whichever applies to the building type indicated on the drawings as well as to the procedures established by the truss plate institute of Canada. Unit stresses used are as per CSA-086-09.
2. Lumber is to be the sizes and grade specified.
3. Moisture content of lumber is not to exceed 19% in service unless otherwise specified.
4. Lumber not to be treated with chemicals unless otherwise specified.
5. Plates shall be applied to both faces of the truss at each joint and shall be positioned as specified.
6. The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals not exceeding 12.5 times in thickness.
7. Where not rigid ceiling is attached directly to the bottom chord, laterally brace the chords at intervals not exceeding 3M (10') o.c.

January 15, 2014

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253644	H16	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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ID:AyrJx8Y?fa2EXF7DRQxSq4zMEz1-7B1MWZCXSt6dvXiAJJZlzl4IAF5MR4YBkldefYwD7f
3x5 II
Scale = 1:59.8



LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
G - B	2x6	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-I	MT20	4.0	9.0	2.00	3.00
C	TTW+p	MT20	3.0	5.0	2.00	Edge
D	TMVW+p	MT20	4.0	6.0	2.00	2.00
E	BMV1+p	MT20	2.0	4.0		
F	BBVWW+p	MT20	5.0	6.0	2.75	3.00
G	BVM1-H	MT20	3.0	6.0	Edge	0.25

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ
G	550	0	550	0
E	451	0	451	0

HANGER BY OTHERS
MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	MAX./MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
G	385	274 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0	0 / 0
E	318	215 / 0	0 / 0	0 / 0	0 / 0	102 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BEARING SIZE FACTOR = 1.15 AT JNT(S) G (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 40	-70.5	-70.5 0.10 (1)	10.00	F-C	0 / 375	0.08 (1)
B-C	-460 / 0	-70.5	-70.5 0.28 (1)	6.25	B-F	0 / 315	0.07 (1)
C-D	-460 / 0	-70.5	-70.5 0.22 (1)	6.25	F-D	0 / 307	0.07 (1)
G-B	-503 / 0	0.0	0.0 0.05 (1)	7.81			
E-D	-408 / 0	0.0	0.0 0.06 (1)	7.81			
G-F	0 / 0	-17.5	-17.5 0.15 (4)	10.00			
F-E	0 / 0	-17.5	-17.5 0.12 (4)	10.00			

TOTAL WEIGHT = 2 X 58 = 116 lb
(M/F)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/999 (0.06")

CSI: TC=0.26 (B-C:1), BC=0.16 (F-G:4),
WB=0.08 (C-F:1), SSI=0.09 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.260 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.58 (C) (INPUT = 0.90)
JSI METAL= 0.12 (D) (INPUT = 1.00)

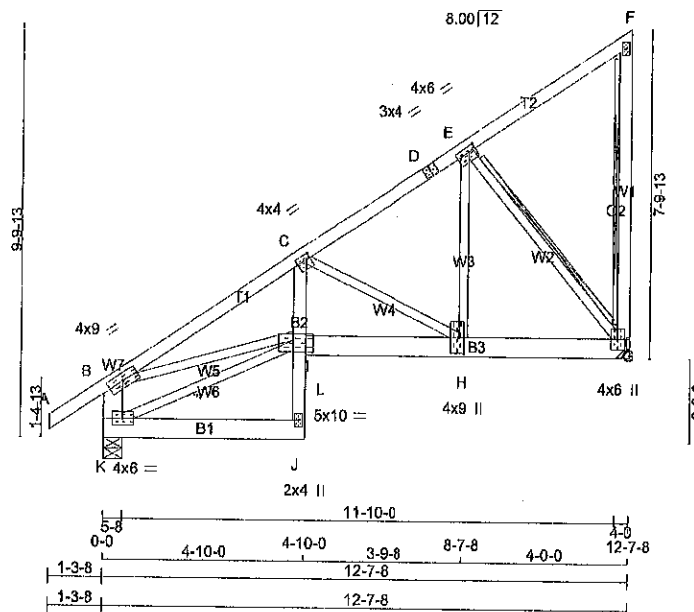


A-ND 73897

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253644	H14	1	2	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MITek Industries, Inc. Mon Jul 20 10:22:29 2015 Page 1
ID:AyrJx9Y7fa2EXF7DRQxSq4zMEz1-f?J_IDBuhZ_mHN7_Jc2eRgXaWvrfCIVN1Wuu475ywD7C
Scale = 1:55.1



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
K - B	2x6 DRY	No.2	SPF		
A - D	2x4 DRY	No.2	SPF		
D - F	2x4 DRY	No.2	SPF		
G - F	2x4 DRY	No.2	SPF		
K - J	2x6 DRY	No.2	SPF		
J - C	2x4 DRY	No.2	SPF		
I - G	2x6 DRY	No.2	SPF		

ALL WEBS 2x3 DRY No.2 SPF
DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
K-B 2	12	TOP
A-D 1	12	TOP
D-F 1	12	TOP
F-G 1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
K-J 2	12	SIDE(90.7)
I-G 2	9	SIDE(268.2)
J-C 1	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
K	2562	0	2562	0
G	3554	0	3554	0

UNFACTORED REACTIONS

	1ST LCASE	MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	1803	1234 / 0	0 / 0	0 / 0	0 / 0	569 / 0	0 / 0
G	2504	1696 / 0	0 / 0	0 / 0	0 / 0	808 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.21 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

2x4 DRY SPF No.2 T-BRACE REQUIRED AT F-G, E-G
FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
K-B	-2083 / 0	0.0 0.0 0.07 (1)	7.81	K-I	-83 / 0	0.02 (1)	
A-B	0 / 27	-70.5 -70.5 0.05 (1)	10.00	B-I	0 / 4093	0.51 (1)	
B-C	-4604 / 0	-70.5 -70.5 0.28 (1)	4.21	C-H	-2183 / 0	0.37 (1)	
C-D	-2577 / 0	-70.5 -70.5 0.12 (1)	5.55	H-E	0 / 3664	0.46 (1)	
D-E	-2577 / 0	-70.5 -70.5 0.12 (1)	5.55	E-G	-3340 / 0	0.69 (1)	
E-F	-18 / 0	-70.5 -70.5 0.09 (1)	6.25				
G-F	-112 / 0	0.0 0.0 0.02 (1)	7.81				
K-J	0 / 77	-198.9 -198.9 0.21 (1)	10.00				
J-I	0 / 496	0.0 0.0 0.16 (1)	10.00				
I-C	0 / 1972	0.0 0.0 0.29 (1)	10.00				
I-L	0 / 4074	-198.9 -198.9 0.59 (1)	10.00				
L-H	0 / 4074	-554.0 -554.0 0.69 (1)	10.00				
H-G	0 / 2152	-554.0 -554.0 0.46 (1)	10.00				

TOTAL WEIGHT = 2 X 75 = 149 lb

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.

LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL	= 21.0 PSF
DL	= 3.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 31.0 PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 10-3-0
END DISTANCE = 5-3-0
END SPAN CARRIED = 10-3-0
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDTL. LOADS BASED ON 56% OF GSL.
(DEFINED BY USER)

*** NON STANDARD GIRDER ***
ADDTL. USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 23.0 P.S.F. G.S.L. PLUS
6.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.42")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (0.42")
CALCULATED VERT. DEFL.(TL) = L/999 (0.12")

CSI: TC=0.26 (B-C:1), BC=0.59 (H-I:1), WB=0.59 (E-G:1), SSI=0.44 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

A-N073896

CONTINUED ON PAGE 2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253644	H14	1	2	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MiTek Industries, Inc. Mon Jul 20 10:22:29 2015 Page 2
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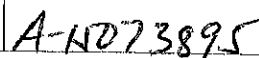
PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-I	MT20	4.0	9.0	
C	TMVW-I	MT20	4.0	4.0	1.50 1.00
D	TS-I	MT20	3.0	4.0	
E	TMWW-I	MT20	4.0	6.0	1.50 2.00
F	TMV+p	MT20	2.0	4.0	
G	BMVW1+p	MT20	4.0	6.0	
H	BMWW-I	MT20	4.0	9.0	4.50 1.50
I	BVMWW-I	MT20	5.0	10.0	3.25 5.50
J	BMV+p	MT20	2.0	4.0	
K	BMVW1-I	MT20	4.0	6.0	

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.
JSI GRIP= 0.88 (E) (INPUT = 0.90)
JSI METAL= 0.41 (B) (INPUT = 1.00)



A-15073896(2)

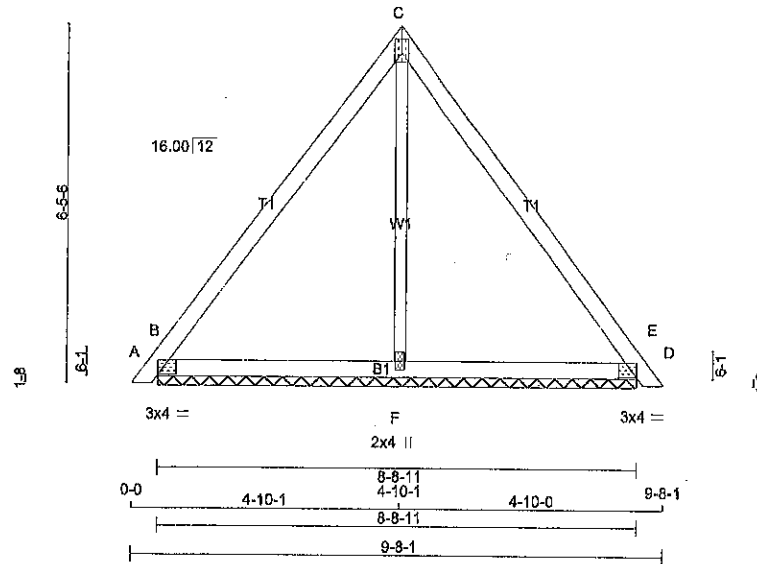




A-ND73894

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 Mitek Industries, Inc. Mon Jul 20 10:22:53 2015 Page 1
ID: AyrJx9Y?fa2EXF7DRQxSq4zMEz1-ycm2LQUy1x0MA3zb9eUGPKXivWzSKUHZ1N1LziywD7f
3x5 II Scale = 1:41.7



TOTAL WEIGHT = 3 X 33 = 99 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B TMB1-I	MT20	3.0	4.0	1.50	2.75
C TTW+p	MT20	3.0	5.0	2.00	Edge
D TMB1-I	MT20	3.0	4.0	1.50	2.75
F BMW1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
B	291	0	291	0	8-8-11	1-8
D	291	0	291	0	8-8-11	1-8
F	234	0	234	0	8-8-11	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	202	156 / 0	0 / 0	0 / 0	0 / 0	45 / 0	0 / 0
D	202	156 / 0	0 / 0	0 / 0	0 / 0	45 / 0	0 / 0
F	171	83 / 0	0 / 0	0 / 0	0 / 0	88 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED				WEBS	
	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX	MAX	CS1 (LC)	MAX. FACTORED FORCE (LBS)	MAX CS1 (LC)
FR-TO		FROM TO					FR-TO	
A-B	0 / 9	-70.5	-70.5	0.01 (1)	10.00		F-C	-139 / 0
B-C	-105 / 0	-70.5	-70.5	0.23 (1)	6.25			0.08 (1)
C-D	-105 / 0	-70.5	-70.5	0.23 (1)	6.25			
D-E	0 / 9	-70.5	-70.5	0.01 (1)	10.00			
B-F	0 / 62	-17.5	-17.5	0.10 (4)	10.00			
F-D	0 / 62	-17.5	-17.5	0.10 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 21.0 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(56 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.23 (B-C:1), BC=0.10 (D-F:4), WB=0.08 (C-F:1), SSI=0.07 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX MIN
MT20	618	354	1087 822 2204 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 6.0 Deg.

JSI GRIP= 0.49 (B) (INPUT = 0.90)
JSI METAL= 0.07 (D) (INPUT = 1.00)

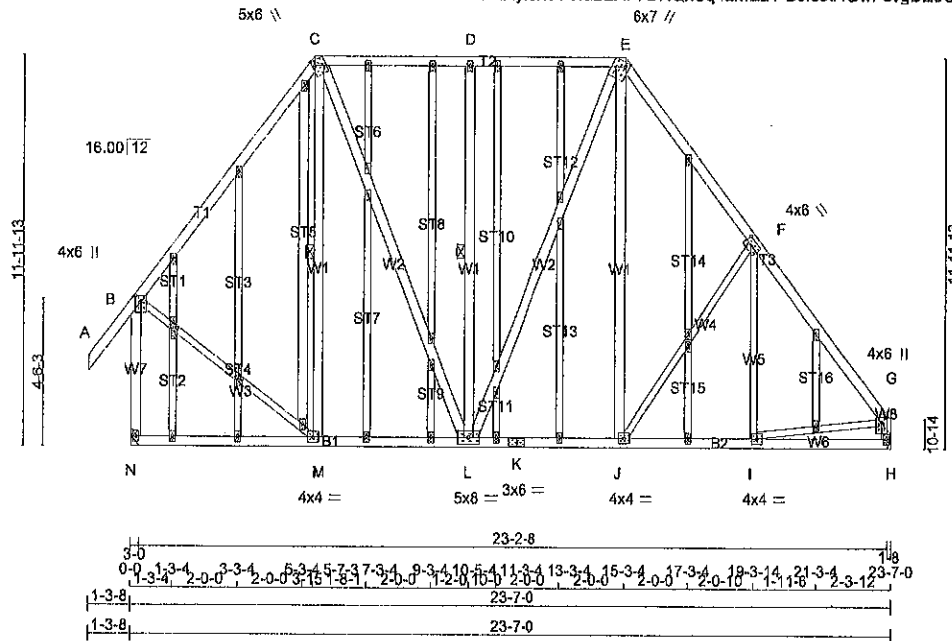


A-ND73893

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253644	H11SG	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 16 2015 Mitek Industries, Inc. Mon Jul 20 10:22:28 2015 Page 1
ID:AyrJx9Y7fa2EXF7DRQxSq4zMEz1-Bolc5tAgwFsvGZocCuXPuS_NQRQbz6ouH_8XbfyWd7f
Scale = 1:7.1.1



TOTAL WEIGHT = 228 lb
(M/F)

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
N - B	2x4	DRY	No.2	SPF	
H - G	2x4	DRY	No.2	SPF	
N - K	2x4	DRY	No.2	SPF	
K - H	2x4	DRY	No.2	SPF	

ALL WEBS EXCEPT	2x4	DRY	No.2	SPF	
J - F	2x3	DRY	No.2	SPF	
I - F	2x3	DRY	No.2	SPF	
B - M	2x3	DRY	No.2	SPF	
I - G	2x3	DRY	No.2	SPF	

ALL GABLE WEBS				
EXCEPT	2x3	DRY	No.2	SPF
ST9	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 24-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TTVW+m	MT20	5.0	6.0	1.75	1.00
D	TMVW+w	MT20	2.0	4.0		
E	TTVW+m	MT20	6.0	7.0	Edge	1.60
F	TMVW-v	MT20	4.0	6.0	2.00	2.50
G	TMVW+p	MT20	4.0	6.0	2.00	2.00
H	BMV1+p	MT20	2.0	4.0		
I, J, M						
I	BMVW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMVW-t	MT20	5.0	8.0		
N	BMV1+p	MT20	2.0	4.0		
O, P, Q, R, S, T, U, V, W, X, Y, Z, AA, AB, AC, AD, AE, AF, AG, AH, AI, AJ, AK, AL, AM, AN, AO, AP, AQ						
O	NP+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
	VERT	HORZ			
N	1177	0	1177	0	0
H	1089	0	1089	0	0

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS				SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	
N	831	554 / 0	0 / 0	0 / 0	0 / 0	277 / 0	0 / 0
H	756	495 / 0	0 / 0	0 / 0	0 / 0	262 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.00FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF C-M, D-L.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM	TO			FR-TO			
A-B	0 / 40	-70.5	-70.5	0.10 (1)	10.00	M-C	-185 / 13	0.14 (1)	
B-C	-682 / 0	-70.5	-70.5	0.31 (1)	6.25	C-L	0 / 417	0.07 (1)	
C-D	-565 / 0	-78.0	-78.0	0.23 (1)	6.25	L-D	-458 / 0	0.36 (1)	
D-E	-665 / 0	-78.0	-78.0	0.23 (1)	6.25	L-E	0 / 156	0.03 (1)	
E-F	-870 / 0	-70.5	-70.5	0.17 (1)	6.25	J-E	0 / 275	0.04 (1)	
F-G	-1028 / 0	-70.5	-70.5	0.18 (1)	8.00	J-F	-235 / 0	0.26 (1)	
N-B	-1136 / 0	0.0	0.0	0.41 (1)	7.45	I-F	0 / 64	0.02 (4)	
H-G	-1037 / 0	0.0	0.0	0.10 (1)	7.71	B-M	0 / 502	0.11 (1)	
						I-G	0 / 638	0.14 (1)	
N-M	0 / 0	-17.5	-17.5	0.13 (4)	10.00				
M-L	0 / 406	-17.5	-17.5	0.16 (4)	10.00				
L-K	0 / 506	-17.5	-17.5	0.12 (1)	10.00				
K-J	0 / 506	-17.5	-17.5	0.12 (1)	10.00				
J-I	0 / 833	-17.5	-17.5	0.14 (1)	10.00				
I-H	0 / 0	-17.5	-17.5	0.07 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD	=	31.0	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(65 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.79")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.79")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.41 (B-N:1), BC=0.16 (L-M:4), WB=0.36 (D-L:1), SS=0.18 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

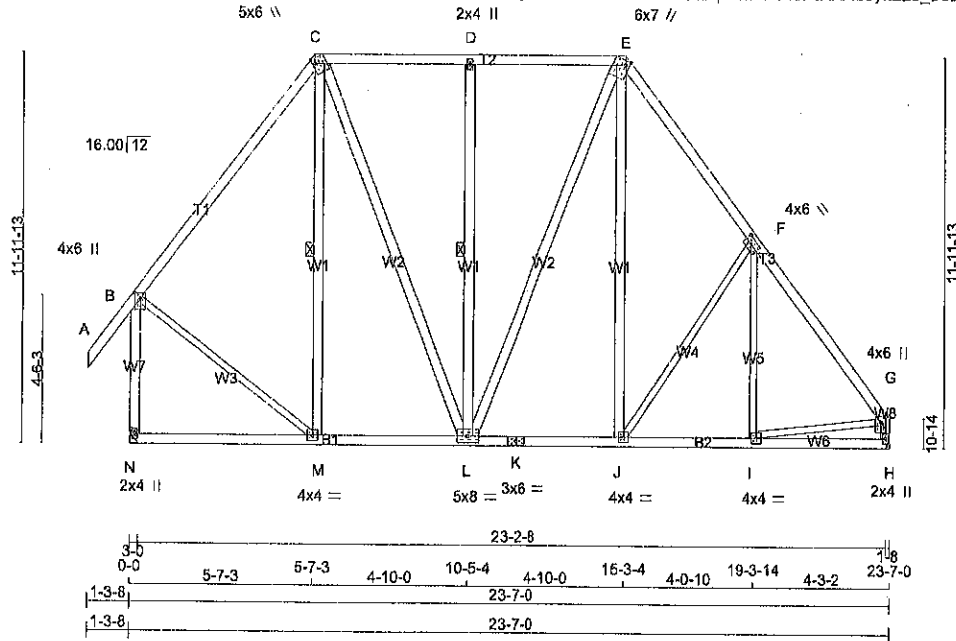
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (AJ) (INPUT = 0.80)
JSI METAL= 0.24 (I) (INPUT = 1.00)



A-N073892

Alpa Roof Truss, Maple



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
N - B	2x4	DRY	No.2
H - G	2x4	DRY	No.2
N - K	2x4	DRY	No.2
K - H	2x4	DRY	No.2
ALL WEBS EXCEPT	2x4	DRY	No.2
J - F	2x3	DRY	No.2
I - F	2x3	DRY	No.2
B - M	2x3	DRY	No.2
I - G	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	4.0	6.0	2.00 2.00
C	TTWW+m	MT20	5.0	6.0	1.76 1.00
D	TMW+w	MT20	2.0	4.0	
E	TTWW+m	MT20	6.0	7.0	Edge 1.50
F	TMWw-l	MT20	4.0	6.0	2.00 2.50
G	TMVW+p	MT20	4.0	6.0	2.00 2.00
H	BMV1+p	MT20	2.0	4.0	
I, J, M					
I	BMWW-l	MT20	4.0	4.0	
K	BS-l	MT20	3.0	6.0	
L	BMWWw-l	MT20	5.0	8.0	
N	BMV1+p	MT20	2.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
 1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT REQRD	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	BRG
N	1177	0	1177	0	0	IN-SX	IN-SX
H	1069	0	1069	0	0	HANGER BY OTHERS	MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	831	554 / 0	0 / 0	0 / 0	0 / 0	277 / 0	0 / 0
H	756	495 / 0	0 / 0	0 / 0	0 / 0	262 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.00FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF C-M, D-L.
 END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	FACTORED FORCE (LBS)	MEMB.	FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 40	M-C	-185 / 13
B-C	-682 / 0	C-L	0 / 417
C-D	-565 / 0	L-D	-466 / 0
D-E	-565 / 0	L-E	0 / 156
E-F	-870 / 0	J-E	0 / 275
F-G	-1028 / 0	J-F	-235 / 0
N-B	-1138 / 0	I-F	0 / 64
H-G	-1037 / 0	B-M	0 / 502
		I-G	0 / 638
N-M	0 / 0		
M-L	0 / 405		
L-K	0 / 505		
K-J	0 / 505		
J-I	0 / 633		
I-H	0 / 0		

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 21.0 PSF
 DL = 3.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55% OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.79")
 CALCULATED VERT. DEFL.(LL)= L/999 (0.02")
 ALLOWABLE DEFL.(TL)= L/360 (0.79")
 CALCULATED VERT. DEFL.(TL)= L/999 (0.04")

CSI: TC=0.41 (B-N:1), BC=0.16 (L-M:4), WB=0.36 (D-L:1), SSI=0.18 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

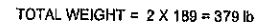
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.67 (I) (INPUT = 0.90)
 JSI METAL= 0.24 (I) (INPUT = 1.00)



A-15073891



A-ND 73890 CONTINUED ON PAGE 2



PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-I	MT20	5.0	8.0	2.00	2.00
C	TTWW-h	MT20	8.0	8.0	Edge	2.75
D	TMWW-I	MT20	4.0	8.0	2.00	2.25
E	TMWW-I	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TS-I	MT20	3.0	6.0		
H	TMWW-I	MT20	4.0	4.0		
I	TMWW-I	MT20	4.0	6.0	2.00	2.25
J	TTWW-h	MT20	8.0	8.0	Edge	2.76
K	TMVW-I	MT20	5.0	8.0	2.00	2.00
M	BMV1+p	MT20	3.0	5.0		
N	BMWW-I	MT20	4.0	6.0	3.00	1.50
O	BMWW-I	MT20	6.0	9.0	4.50	2.25
P	BMWW-I	MT20	4.0	4.0		
Q	BS-I	MT20	4.0	6.0		
R	BMWWW-I	MT20	5.0	6.0		
S	BS-I	MT20	4.0	6.0		
T	BMWW-I	MT20	4.0	4.0		
U	BMWW-I	MT20	8.0	9.0	4.50	2.25
V	BMWW-I	MT20	4.0	8.0	3.00	1.50
W	BMV1+p	MT20	3.0	5.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 114.5 lbs FACTORED DOWN AT 2-2-12 ON TOP CHORD, AND 1833.8 lbs FACTORED DOWN AT 4-10-8, AND 3554.0 lbs FACTORED DOWN AT 28-8-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

- 1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	618	354	1687
			822
			2284
			1656

PLATE PLACEMENT TOL. = 0.250 inches

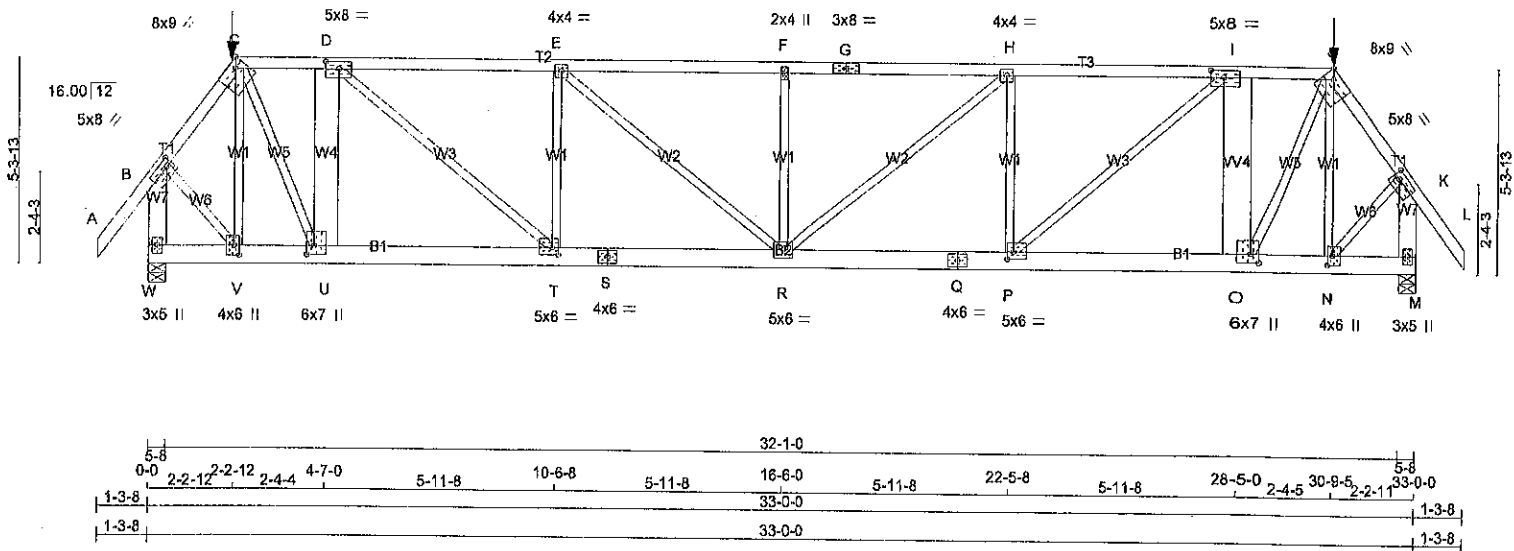
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.68 (T) (INPUT = 0.90)

JSI METAL= 0.61 (Q) (INPUT = 1.00)



A-10073890(2)



LUMBER				N. L. G. A. RULES	
CHORDS	SIZE	LUMBER	DESCR.		
A - C	2x4	DRY	No.2	SPF	
C - G	2x4	DRY	1650F 1.5E	SPF	
G - J	2x4	DRY	1650F 1.5E	SPF	
J - L	2x4	DRY	No.2	SPF	
M - B	2x6	DRY	No.2	SPF	
M - K	2x6	DRY	No.2	SPF	
W - S	2x6	DRY	No.2	SPF	
S - Q	2x6	DRY	No.2	SPF	
Q - M	2x6	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
U - D	2x8	DRY	No.2	SPF	
O - I	2x8	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMWW-h	MT20	5.0	8.0 2.00 2.00
C	TTWW-h	MT20	8.0	8.0 Edge 2.75
D	TMWW-t	MT20	5.0	8.0 2.25 4.00
E	TMWW-t	MT20	4.0	4.0
F	TMWW-w	MT20	2.0	4.0
G	TS-t	MT20	3.0	8.0
H	TMWW-t	MT20	4.0	4.0
I	TMWW-t	MT20	6.0	8.0 2.25 4.00
J	TTWW-h	MT20	8.0	8.0 Edge 2.75
K	TMWW-t	MT20	5.0	8.0 2.00 2.00
M	BMV1-p	MT20	3.0	5.0
N	BMWW-h	MT20	4.0	6.0 3.00 1.50
O	BMWW-h	MT20	6.0	7.0 2.75 2.25
P	BMWW-t	MT20	5.0	6.0 2.25 2.00
Q	BS-t	MT20	4.0	6.0
R	BMWWW-t	MT20	5.0	6.0
S	BS-t	MT20	4.0	6.0
T	BMWW-t	MT20	5.0	6.0 2.25 2.00
U	BMWW-h	MT20	6.0	7.0 2.75 2.25
V	BMWW-t	MT20	4.0	6.0 3.00 1.50
W	BMV1-p	MT20	3.0	5.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
	FACTORED	MAXIMUM FACTORED	INPUT	REQD					
	GROSS REACTION	GROSS REACTION	BRG	BRG					
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX		
W	2919	0	2919	0	0	5-8	4-0		
M	2919	0	2919	0	0	5-8	4-0		

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN	COMPONENT REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
W	2055	1400 / 0	0 / 0	0 / 0	0 / 0	855 / 0	0 / 0		
M	2055	1400 / 0	0 / 0	0 / 0	0 / 0	855 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.83FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING				
TOTAL LOAD CASES: (4)				
CHORDS				
MEMB.	FORCE	FACTORED	MAX	MAX
	(LBS)	VERT. LOAD LC1	MAX	UNBRAC
		(PLF)	CSI (LC)	LENGTH
FR-TO		FROM	TO	FR-TO
A-B	0 / 40	-70.5	-70.5 0.11 (1)	10.00
B-C	-2166 / 0	-70.5	-70.5 0.13 (1)	4.50
C-D	-2358 / 0	-138.7	-138.7 0.86 (1)	4.47
D-E	-4264 / 0	-138.7	-138.7 0.86 (1)	3.08
E-F	-4859 / 0	-138.7	-138.7 0.94 (1)	2.83
F-G	-4859 / 0	-138.7	-138.7 0.94 (1)	2.83
G-H	-4859 / 0	-138.7	-138.7 0.94 (1)	2.83
H-I	-4263 / 0	-138.7	-138.7 0.86 (1)	3.08
I-J	-2358 / 0	-138.7	-138.7 0.42 (1)	4.47
J-K	-2156 / 0	-70.5	-70.5 0.13 (1)	4.50
K-L	0 / 40	-70.5	-70.5 0.11 (1)	10.00
W-B	-2943 / 0	0.0	0.0 0.26 (1)	6.08
M-K	-2943 / 0	0.0	0.0 0.26 (1)	6.08
W-V	0 / 0	-34.5	-34.5 0.11 (1)	10.00
V-U	0 / 1263	-34.5	-34.5 0.27 (1)	10.00
U-T	0 / 2360	-34.5	-34.5 0.39 (1)	10.00
T-S	0 / 4264	-34.5	-34.5 0.86 (1)	10.00
S-R	0 / 4264	-34.5	-34.5 0.86 (1)	10.00
R-Q	0 / 4263	-34.5	-34.5 0.86 (1)	10.00
Q-P	0 / 4263	-34.5	-34.5 0.86 (1)	10.00
P-O	0 / 2360	-34.5	-34.5 0.39 (1)	10.00
O-N	0 / 1263	-34.5	-34.5 0.27 (1)	10.00
N-M	0 / 0	-34.5	-34.5 0.11 (1)	10.00

FACTORED CONCENTRATED LOADS (LBS)					
JT	LOC.	LC1	MAX-	MAX+	FACE
C	2-2-12	-115	-115	---	FRONT
J	30-9-5	-114	-114	---	FRONT

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 2-2-12
RIGHT SETBACK = 2-2-11
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDTL LOADS BASED ON 66 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

66 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/999 (0.23")
ALLOWABLE DEFL.(TL) = L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/940 (0.42")

CSI: TC=0.94 (E-F:1), BC=0.65 (R-T:1), WB=0.83
(J-O:1), SS=0.37 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN
THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1657 822 2294 1656

PLATE PLACEMENT TOL. = 0.260 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (J) (INPUT = 0.90)
JSI METAL= 0.86 (Q) (INPUT = 1.00)

A-11073889



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253644	H01	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MiTek Industries, Inc. Mon Jul 20 10:22:26 2015 Page 2
ID:AyrJx9Y?fa2EXF7DRQxSq4zMEz1-mE3TSr8OdKUKpIqDVVmzhGgMktEGNmIrrB0wt_KywD7S

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 114.2 lbs. FACTORED DOWN AT 30-9-5, AND 114.5 lbs. FACTORED DOWN AT 2-2-12 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

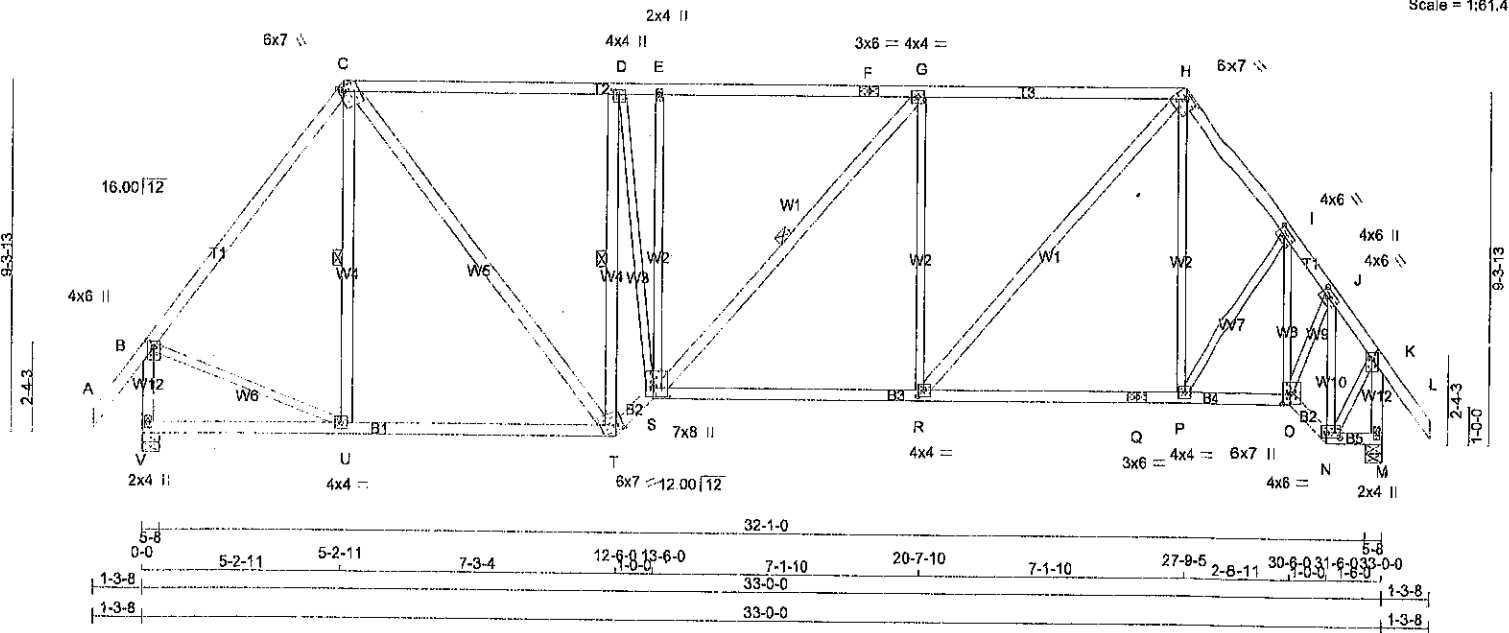
NOTES- (1)

- 1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.



A-15073889(2)

Alpa Roof Truss, Maple



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	DESCR.
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - L	2x4	DRY	No.2
V - B	2x4	DRY	No.2
M - K	2x4	DRY	No.2
V - T	2x4	DRY	No.2
T - S	2x4	DRY	No.2
S - Q	2x4	DRY	No.2
Q - O	2x4	DRY	No.2
O - N	2x4	DRY	No.2
N - M	2x4	DRY	No.2
ALL WEBS 2x3 DRY No.2 SPF			
EXCEPT			
U - C	2x4	DRY	No.2
C - T	2x4	DRY	No.2
T - D	2x4	DRY	No.2
S - G	2x4	DRY	No.2
R - H	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	4.0	6.0	2.00 2.00
C	TTWW+m	MT20	6.0	7.0	Edge 1.50
D	TMVW+t	MT20	4.0	4.0	1.50 1.75
E	TMVW+w	MT20	2.0	4.0	
F	TS-i	MT20	3.0	6.0	
G	TMVW-i	MT20	4.0	4.0	
H	TTWW-h	MT20	6.0	7.0	1.50 3.75
I	TMVW-l	MT20	4.0	6.0	2.00 2.50
J	TMVW-l	MT20	4.0	6.0	2.00 2.50
K	TMVW+p	MT20	4.0	6.0	2.00 2.00
M	BMV1+p	MT20	2.0	4.0	
N	BBWW-i	MT20	4.0	6.0	1.75 4.25
O	BBWW+p	MT20	6.0	7.0	3.25 2.75
P	BMVW-i	MT20	4.0	4.0	
Q	BS-i	MT20	3.0	6.0	
R	BMVW-i	MT20	4.0	4.0	2.00 1.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS					
JT	FACTORED GROSS REACTION	FACTORED GROSS REACTION	INPUT BRG	REQD BRG	
V	VERT	HORZ	DOWN	HORZ	UPLIFT
M	1548	0	1548	0	0

UNFACTORED REACTIONS						
JT	1ST CASE	MAX./MIN.	COMPONENT	REACTIONS		
V	1089	750 / 0	0 / 0	0 / 0	0 / 0	338 / 0
M	1081	753 / 0	0 / 0	0 / 0	0 / 0	339 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.42FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF C-U, D-T, G-S.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING									
TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 (LC)	MAX. UNBRACED LENGTH (FR-TO)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FR-TO)	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FR-TO)
A-B	0 / 40	-70.5	-70.5	0.10 (1)	10.00	U-C	-129 / 67	0.05 (1)	0.05 (1)
B-C	-1210 / 0	-70.5	-70.5	0.46 (1)	5.34	C-T	0 / 909	0.15 (1)	0.15 (1)
C-D	-1294 / 0	-70.5	-70.5	0.71 (1)	4.68	T-D	-1957 / 0	0.80 (1)	0.80 (1)
D-E	-1465 / 0	-70.5	-70.5	0.60 (1)	4.42	D-S	0 / 1338	0.30 (1)	0.30 (1)
E-F	-1464 / 0	-70.5	-70.5	0.68 (1)	4.60	S-E	0 / 37	0.01 (1)	0.01 (1)
F-G	-1464 / 0	-70.5	-70.5	0.68 (1)	4.60	S-G	-3 / 16	0.00 (1)	0.00 (1)
G-H	-1453 / 0	-70.5	-70.5	0.68 (1)	4.52	R-G	-594 / 0	0.81 (1)	0.81 (1)
H-I	-1354 / 0	-70.5	-70.5	0.09 (1)	5.51	R-H	0 / 981	0.16 (1)	0.16 (1)
I-J	-1255 / 0	-70.5	-70.5	0.08 (1)	5.68	P-H	0 / 98	0.03 (4)	0.03 (4)
J-K	-896 / 0	-70.5	-70.5	0.10 (1)	8.25	P-I	0 / 29	0.01 (4)	0.01 (4)
K-L	0 / 40	-70.5	-70.5	0.10 (1)	10.00	O-I	-288 / 0	0.09 (1)	0.09 (1)
V-B	-1518 / 0	0.0	0.0	0.19 (1)	6.87	O-J	0 / 824	0.19 (1)	0.19 (1)
M-K	-1539 / 0	0.0	0.0	0.20 (1)	6.83	N-J	-1102 / 0	0.29 (1)	0.29 (1)
V-U	0 / 0	-17.5	-17.5	0.21 (4)	10.00	B-U	0 / 769	0.17 (1)	0.17 (1)
U-T	0 / 721	-17.5	-17.5	0.27 (4)	10.00	N-K	0 / 822	0.19 (1)	0.19 (1)
T-S	0 / 1868	-17.5	-17.5	0.30 (1)	10.00				
S-R	0 / 1454	-17.5	-17.5	0.36 (1)	10.00				
R-Q	0 / 799	-17.5	-17.5	0.29 (4)	10.00				
Q-P	0 / 799	-17.5	-17.5	0.29 (4)	10.00				
P-O	0 / 783	-17.5	-17.5	0.19 (1)	10.00				
O-N	0 / 671	-17.5	-17.5	0.11 (1)	10.00				
N-M	0 / 0	-17.5	-17.5	0.01 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/999 (0.10")
ALLOWABLE DEFL.(TL) = L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/999 (0.22")

CSI: TC=0.71 (C-D:1), BC=0.36 (R-S:1), WB=0.81 (G-R:1), SS=0.25 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP(DRY)	SHEAR	SECTION	(PSI)	(PLI)
	MAX	MIN	MAX	MIN	MAX
MT20	618	354	1667	822	2284

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (K) (INPUT = 0.90)
JSI METAL= 0.29 (B) (INPUT = 1.00)

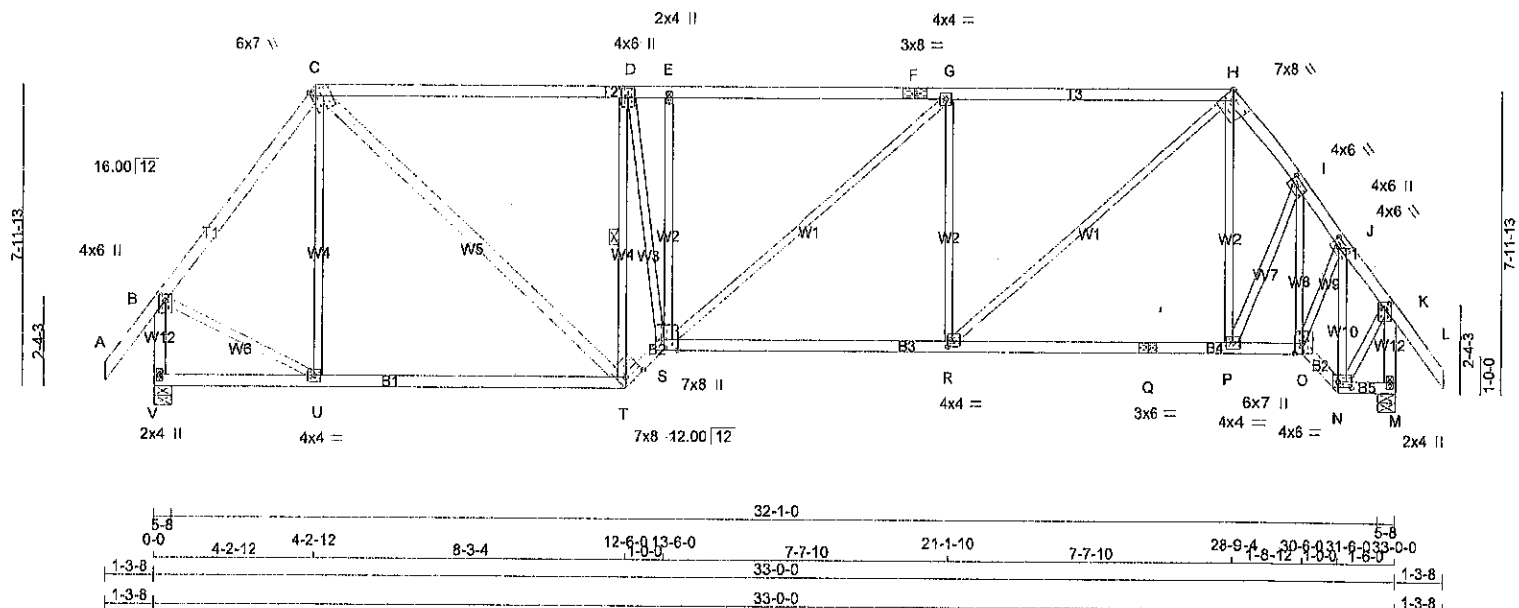


A-15073888

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253643	H03T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MiTek Industries, Inc. Mon Jul 20 08:35:11 2016 Page 1
ID:AyrJx9Y7fa2EXF7DRQXsq4zMEz1-9R196rHZrUitEw_yfcVYUv0uFAeQjGkQ50MQAFywEi
Scale = 1:61.4



LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - C	2x4	DRY No.2	SPF	
C - F	2x4	DRY 1850F 1.5E	SPF	
F - H	2x4	DRY 1850F 1.5E	SPF	
H - L	2x4	DRY No.2	SPF	
V - B	2x4	DRY No.2	SPF	
M - K	2x4	DRY No.2	SPF	
V - T	2x4	DRY No.2	SPF	
T - S	2x4	DRY No.2	SPF	
S - Q	2x4	DRY No.2	SPF	
Q - O	2x4	DRY No.2	SPF	
O - N	2x4	DRY No.2	SPF	
N - M	2x4	DRY No.2	SPF	
ALL WEBS	2x3	DRY No.2	SPF	
EXCEPT				
C - T	2x4	DRY No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TTWW+m	MT20	6.0	7.0	Edge	1.50
D	TMWW+t	MT20	4.0	6.0	3.00	1.75
E	TMWW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMWW-l	MT20	4.0	4.0		
H	TTWW-h	MT20	7.0	8.0	Edge	2.75
I	TMWW-i	MT20	4.0	6.0	2.00	2.60
J	TMWW-l	MT20	4.0	6.0	2.00	2.60
K	TMVW+p	MT20	4.0	6.0	2.00	2.00
M	BMV1+p	MT20	2.0	4.0		
N	BBWW-l	MT20	4.0	6.0	1.75	4.25
O	BBWW+p	MT20	6.0	7.0	3.25	2.75
P	BMWW-t	MT20	4.0	4.0		
Q	BS-t	MT20	3.0	6.0		
R	BMWW-l	MT20	4.0	4.0	1.75	1.50
S	BBWW+p	MT20	7.0	8.0	3.25	4.25
T	BBWW-h	MT20	7.0	8.0	2.25	5.50
U	BMWW-l	MT20	4.0	4.0		
V	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	VERT	HORZ	GROSS REACTION	DOWN	HORZ	BRG	IN-SX	BRG	IN-SX
V	1548	0	1548	0	0	5-8	1-11		
M	1653	0	1653	0	0	5-8	1-11		

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	1089	760 / 0	0 / 0	0 / 0	0 / 0	338 / 0	0 / 0
M	1091	753 / 0	0 / 0	0 / 0	0 / 0	339 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.63FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF D-T.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX	CS1 (LC)	MEMB.	FORCE (LBS)	MAX	CS1 (LC)
FR-TO		FROM TO				FR-TO			
A-B	0 / 40	-70.5	-70.5	0.10 (1)	10.00	U-C	-179 / 56	0.22 (1)	
B-C	-1200 / 0	-70.5	-70.5	0.27 (1)	5.55	C-T	0 / 1115	0.18 (1)	
C-D	-1634 / 0	-70.5	-70.5	0.77 (1)	4.80	T-D	-2245 / 0	0.90 (1)	
D-E	-1764 / 0	-70.5	-70.5	0.64 (1)	4.53	D-S	0 / 1505	0.34 (1)	
E-F	-1762 / 0	-70.5	-70.5	0.65 (1)	4.79	S-E	0 / 104	0.02 (1)	
F-G	-1762 / 0	-70.5	-70.5	0.65 (1)	4.79	S-G	0 / 51	0.01 (1)	
G-H	-1723 / 0	-70.5	-70.5	0.65 (1)	4.83	R-G	-854 / 0	0.55 (1)	
H-I	-1366 / 0	-70.5	-70.5	0.05 (1)	5.52	R-H	0 / 1217	0.27 (1)	
I-J	-1268 / 0	-70.5	-70.5	0.05 (1)	5.68	P-H	0 / 76	0.03 (4)	
J-K	-895 / 0	-70.5	-70.5	0.10 (1)	6.25	P-I	0 / 77	0.02 (1)	
K-L	0 / 40	-70.5	-70.5	0.10 (1)	10.00	O-I	-266 / 0	0.09 (1)	
V-B	-1535 / 0	0.0	0.0	0.20 (1)	6.64	O-J	0 / 802	0.18 (1)	
M-K	-1639 / 0	0.0	0.0	0.20 (1)	6.63	N-J	-1106 / 0	0.29 (1)	
						B-U	0 / 786	0.18 (1)	
						N-K	0 / 825	0.19 (1)	
V-U	0 / 0	-17.5	-17.5	0.25 (4)	10.00				
U-T	0 / 714	-17.5	-17.5	0.31 (4)	10.00				
T-S	0 / 2205	-17.5	-17.5	0.36 (1)	10.00				
S-R	0 / 1724	-17.5	-17.5	0.42 (1)	10.00				
R-Q	0 / 806	-17.5	-17.5	0.32 (4)	10.00				
Q-P	0 / 806	-17.5	-17.5	0.32 (4)	10.00				
P-O	0 / 777	-17.5	-17.5	0.21 (1)	10.00				
O-N	0 / 673	-17.5	-17.5	0.11 (1)	10.00				
N-M	0 / 0	-17.5	-17.5	0.01 (4)	10.00				

TOTAL WEIGHT = 172 lb

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 21.0 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(65 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/999 (0.13")
ALLOWABLE DEFL.(TL) = L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/999 (0.28")

CSI: TC=0.77 (C-D:1), BC=0.42 (R-S:1), WB=0.80 (D-T:1), SSI=0.31 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 822 2284 1856

PLATE PLACEMENT TOL. = 0.250 inches

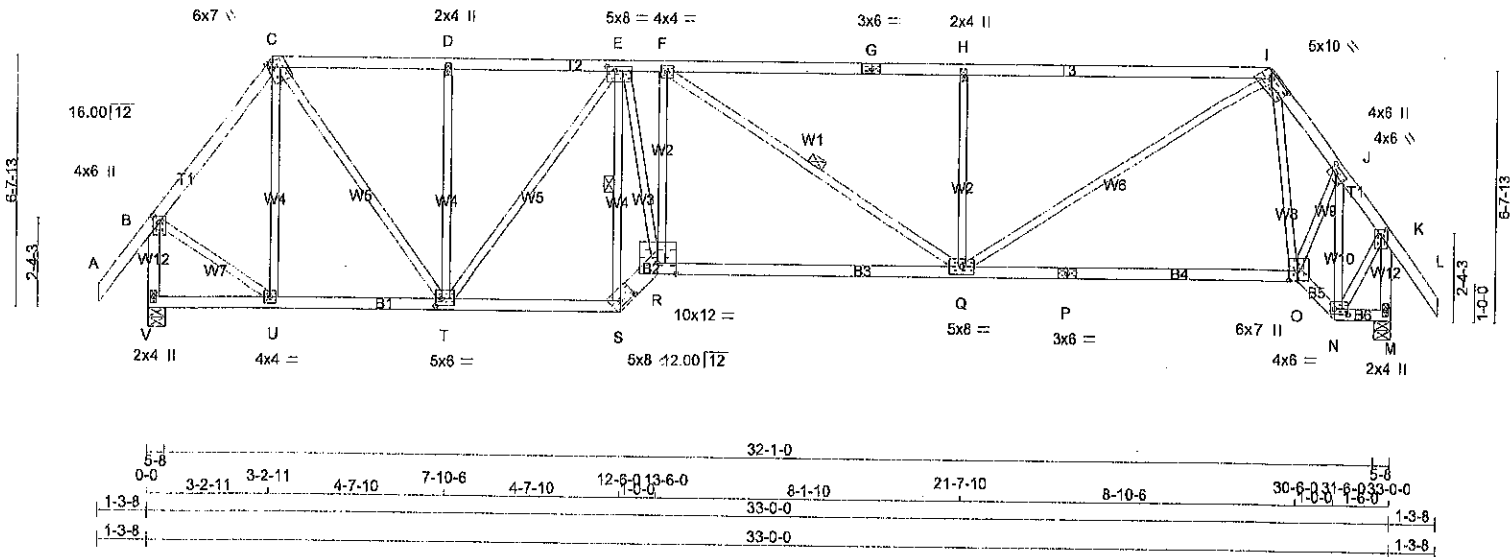
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (K) (INPUT = 0.90)
JSI METAL= 0.35 (T) (INPUT = 1.00)

A-1073887



Alpha Roof Truss, Maple



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - G	2x4	DRY	1650F 1.5E
G - I	2x4	DRY	1650F 1.5E
I - L	2x4	DRY	No.2
V - B	2x4	DRY	No.2
M - K	2x4	DRY	No.2
V - S	2x4	DRY	No.2
S - R	2x4	DRY	No.2
R - P	2x4	DRY	No.2
P - O	2x4	DRY	No.2
O - N	2x4	DRY	No.2
N - M	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	4.0	6.0	2.00	2.00
C	TTWW+m	MT20	6.0	7.0	Edge	1.60
D	TMWW+w	MT20	2.0	4.0		
E	TMWWW-l	MT20	6.0	8.0	1.50	2.50
F	TMWW-l	MT20	4.0	4.0		
G	TS-l	MT20	3.0	8.0		
H	TMWW+w	MT20	2.0	4.0		
I	TTWW-h	MT20	6.0	10.0	1.50	6.75
J	TMWW-l	MT20	4.0	6.0	2.00	2.50
K	TMWW+p	MT20	4.0	6.0	2.00	2.00
M	BMV1+p	MT20	2.0	4.0		
N	BBWW-l	MT20	4.0	6.0	1.75	4.50
O	BBWW+p	MT20	6.0	7.0	3.25	2.75
P	BS-l	MT20	3.0	8.0		
Q	BMWWW-l	MT20	5.0	8.0		
R	BBWW-l	MT20	10.0	12.0	3.25	6.00
S	BBWW-h	MT20	6.0	8.0	1.75	5.00
T	BMWWW-l	MT20	5.0	6.0	2.25	2.00
U	BMWW-l	MT20	4.0	4.0		
V	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	VERT	GROSS REACTION	HORIZ	GROSS REACTION	HORIZ	BRG	IN-SX	BRG	IN-SX
V	1548	0	1548	0	0	5-8	1-11		
M	1553	0	1553	0	0	5-8	1-11		

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERMLIVE	WIND	DEAD	SOIL
V	1089	750 / 0	0 / 0	0 / 0	0 / 0	338 / 0	0 / 0
M	1091	753 / 0	0 / 0	0 / 0	0 / 0	339 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.28FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF E-S, F-Q.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	MAX. UNBRACED LENGTH (LC)
FR-TO		FROM TO		FR-TO			
A-B	0 / 40	-70.5 -70.5 0.10 (1)	10.00	U-C	-319 / 0	0.23 (1)	
B-C	-1141 / 0	-70.5 -70.5 0.15 (1)	5.78	C-T	0 / 1211	0.27 (1)	
C-D	-1391 / 0	-70.5 -70.5 0.19 (1)	5.90	T-D	-353 / 0	0.26 (1)	
D-E	-1391 / 0	-70.5 -70.5 0.19 (1)	5.90	T-E	-708 / 0	0.87 (1)	
E-F	-2230 / 0	-70.5 -70.5 0.28 (1)	4.82	S-E	-1694 / 0	0.48 (1)	
F-G	-2130 / 0	-70.5 -70.5 0.79 (1)	4.29	E-R	0 / 2230	0.60 (1)	
G-H	-2130 / 0	-70.5 -70.5 0.79 (1)	4.29	R-F	-323 / 0	0.16 (1)	
H-I	-1219 / 0	-70.5 -70.5 0.80 (1)	4.29	F-Q	-129 / 0	0.09 (1)	
I-J	-1276 / 0	-70.5 -70.5 0.07 (1)	5.65	Q-H	-654 / 0	0.32 (1)	
J-K	-894 / 0	-70.5 -70.5 0.10 (1)	6.25	Q-I	0 / 1611	0.36 (1)	
K-L	0 / 40	-70.5 -70.5 0.10 (1)	10.00	I-O	-158 / 5	0.07 (1)	
V-B	-1529 / 0	0.0 0.0 0.20 (1)	6.65	O-J	0 / 762	0.17 (1)	
M-K	-1539 / 0	0.0 0.0 0.20 (1)	6.63	N-J	-1112 / 0	0.29 (1)	
				B-U	0 / 788	0.18 (1)	
				N-K	0 / 829	0.19 (1)	
V-U	0 / 0	-17.5 -17.5 0.06 (4)	10.00				
U-T	0 / 674	-17.5 -17.5 0.15 (1)	10.00				
T-S	0 / 1801	-17.5 -17.5 0.34 (1)	10.00				
S-R	0 / 2507	-17.5 -17.5 0.30 (1)	10.00				
R-Q	0 / 2237	-17.5 -17.5 0.55 (1)	10.00				
Q-P	0 / 780	-17.5 -17.5 0.42 (4)	10.00				
P-O	0 / 780	-17.5 -17.5 0.42 (4)	10.00				
O-N	0 / 677	-17.5 -17.5 0.11 (1)	10.00				
N-M	0 / 0	-17.5 -17.5 0.01 (4)	10.00				

TOTAL WEIGHT = 162 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 21.0 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/999 (0.14")
ALLOWABLE DEFL.(TL) = L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/999 (0.32")

CSI: TC=0.80 (H-I:1), BC=0.55 (Q-R:1), WB=0.87 (E-T:1), SS=0.27 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

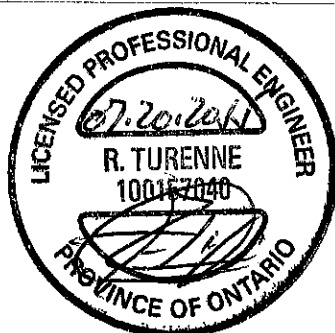
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

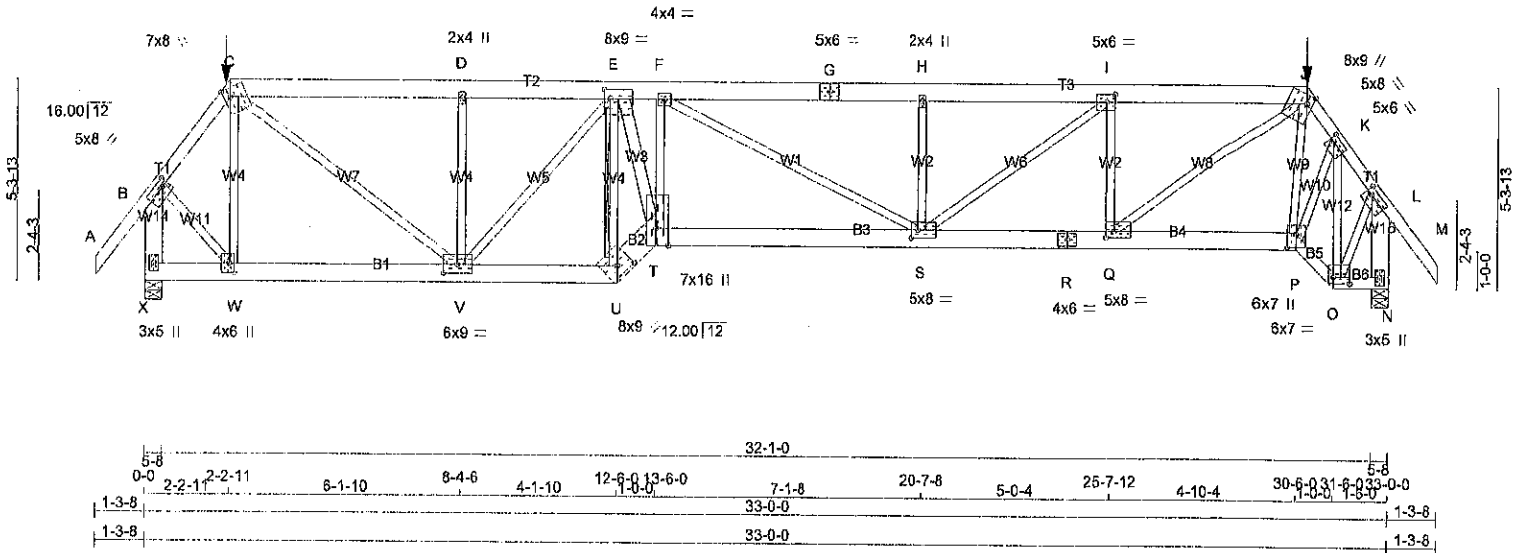
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (K) (INPUT = 0.90)
JSI METAL= 0.45 (E) (INPUT = 1.00)



A-1073886

Alpa Roof Truss, Maple



LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x6	DRY	No.2	SPF
G - J	2x6	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF
X - B	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
X - U	2x6	DRY	No.2	SPF
U - T	2x8	DRY	No.2	SPF
T - R	2x6	DRY	1650F 1.5E	SPF
R - P	2x6	DRY	1650F 1.5E	SPF
P - O	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
E - T	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-I	MT20	5.0	8.0	2.00	2.00
C	TTWW+m	MT20	7.0	8.0	2.50	2.00
D	TMVW+w	MT20	2.0	4.0		
E	TMVWW-I	MT20	8.0	9.0	3.00	2.00
F	TMVW-I	MT20	4.0	4.0		
G	TS-I	MT20	5.0	6.0		
H	TMVW+w	MT20	2.0	4.0		
I	TMVW-I	MT20	5.0	6.0	2.50	2.75
J	TTWW+m	MT20	8.0	8.0	2.75	1.75
K	TMVW-I	MT20	5.0	6.0	2.00	1.50
L	TMVW-I	MT20	5.0	8.0	2.00	2.00
N	BMV-I+p	MT20	3.0	5.0		
O	BBW-I	MT20	6.0	7.0	2.00	5.50
P	BBWV+p	MT20	6.0	7.0	4.00	3.00
Q	BMVW-I	MT20	5.0	8.0	2.00	2.50
R	BS-I	MT20	4.0	6.0		
S	BMVWW-I	MT20	5.0	8.0	2.50	2.25
T	BBWV+p	MT20	7.0	18.0	Edge 4.00	
U	BBW-h	MT20	8.0	9.0	3.00	4.50
V	BMVWW-I	MT20	8.0	9.0	2.75	4.25

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	UPLIFT	IN-SX
X	2919	0	0	5-8
N	2906	0	0	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX. MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
X	2055	1400 / 0	0 / 0	0 / 0	0 / 0	655 / 0	0 / 0
N	2046	1393 / 0	0 / 0	0 / 0	0 / 0	654 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.20FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

2x4 DRY SPF No.2 T-BRACE REQUIRED AT E-U
FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 40	-70.5 -70.5	0.11 (1)	10.00	W-C	-775 / 0	0.29 (1)
B-C	-2170 / 0	-70.5 -70.5	0.13 (1)	4.49	C-V	0 / 3148	0.78 (1)
C-D	-3753 / 0	-138.7 -138.7	0.48 (1)	3.94	V-D	-837 / 0	0.32 (1)
D-E	-3753 / 0	-138.7 -138.7	0.39 (1)	4.03	V-E	-1452 / 0	0.99 (1)
E-F	-6089 / 0	-138.7 -138.7	0.32 (1)	3.36	U-E	-4366 / 0	0.95 (1)
F-G	-5883 / 0	-70.5 -70.5	0.56 (1)	3.20	E-T	0 / 5337	0.94 (1)
G-H	-5883 / 0	-70.5 -70.5	0.56 (1)	3.20	T-F	-117 / 48	0.03 (1)
H-I	-5883 / 0	-70.5 -70.5	0.41 (1)	3.33	F-S	-238 / 0	0.31 (1)
I-J	-4289 / 0	-70.5 -70.5	0.28 (1)	3.86	S-H	-478 / 0	0.12 (1)
J-K	-2591 / 0	-70.5 -70.5	0.14 (1)	4.15	S-I	0 / 2011	0.50 (1)
K-L	-1751 / 0	-70.5 -70.5	0.14 (1)	4.88	Q-I	-1509 / 0	0.40 (1)
L-M	0 / 40	-70.5 -70.5	0.11 (1)	10.00	P-J	-315 / 0	0.08 (1)
X-B	-2982 / 0	0.0 0.0	0.28 (1)	6.07	O-K	-2252 / 0	0.62 (1)
N-L	-2880 / 0	0.0 0.0	0.27 (1)	6.13	B-W	0 / 1669	0.41 (1)
X-W	0 / 0	-34.5 -34.5	0.14 (1)	10.00	O-L	0 / 1651	0.41 (1)
W-V	0 / 1283	-34.5 -34.5	0.31 (1)	10.00	P-K	0 / 1660	0.41 (1)
V-U	0 / 4682	-34.5 -34.5	0.70 (1)	10.00	Q-J	0 / 3428	0.85 (1)
U-T	0 / 6484	-34.5 -34.5	0.69 (1)	10.00			
T-S	0 / 6092	-102.7 -102.7	0.74 (1)	10.00			
S-R	0 / 4289	-102.7 -102.7	0.53 (1)	10.00			
R-Q	0 / 4289	-102.7 -102.7	0.53 (1)	10.00			
Q-P	0 / 1548	-102.7 -102.7	0.25 (1)	10.00			
P-O	0 / 1374	-34.5 -34.5	0.25 (1)	10.00			
O-N	0 / 0	-34.5 -34.5	0.02 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-2-11	-114	-114	---	FRONT	VERT	TOTAL
J	30-9-5	-114	-114	---	FRONT	VERT	TOTAL

TOTAL WEIGHT = 186 lb

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	21.0	PSF
DL	=	3.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.0	PSF
TOTAL LOAD	=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHlp
SIDE SETBACK = 2-2-11
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDTL LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 13-6-0 OF SPAN MEASURED FROM THE LEFT.

GIRDER TYPE: CStdGrlder
START DISTANCE = 13-6-0
START SPAN CARRIED = 5-10-8
END DISTANCE = 30-6-0
END SPAN CARRIED = 5-10-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDTL LOADS BASED ON 55 % OF GSL.
(DEFINED BY USER)

GIRDER TYPE: CPrimeHlp
SIDE SETBACK = 2-2-11
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDTL LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 2-6-0 OF SPAN MEASURED FROM THE RIGHT.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/899 (0.36")
ALLOWABLE DEFL.(TL) = L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/608 (0.65")

CSI: TC=0.66 (F-H:1), BC=0.74 (S-T:1), WB=0.99 (E-V:1), SF=0.92 (O-P:1)



4-1013 885
CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253643	H01T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MITek Industries, Inc. Mon Jul 20 08:35:09 2015 Page 2
ID:AyrJx9Y?fa2EXF7DRQxSq4zMEz1-D2AOH9FJJtdWghpGV5WQQbpbzrf6cox7dlJ5NywEk

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
W	BMWWH	MT20	4.0	6.0	3.00	1.50
X	BMV1+p	MT20	3.0	5.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 114.2 lbs FACTORED DOWN AT 2-2-11,
AND 114.2 lbs FACTORED DOWN AT 30-9-5 ON
TOP CHORD. DESIGN FOR UNSPECIFIED
CONNECTION(S) IS DELEGATED TO THE
BUILDING DESIGNER.

NOTES- (1)

- Lateral brace(s) shown shall be 1x4 for Part 9 design as per
OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN
THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	618	354	1667 822 2284 1656

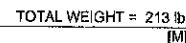
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

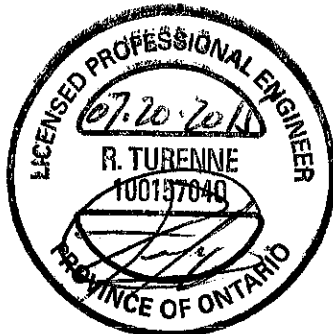
JSI GRIP= 0.89 (Q) (INPUT = 0.90)
JSI METAL= 0.85 (R) (INPUT = 1.00)

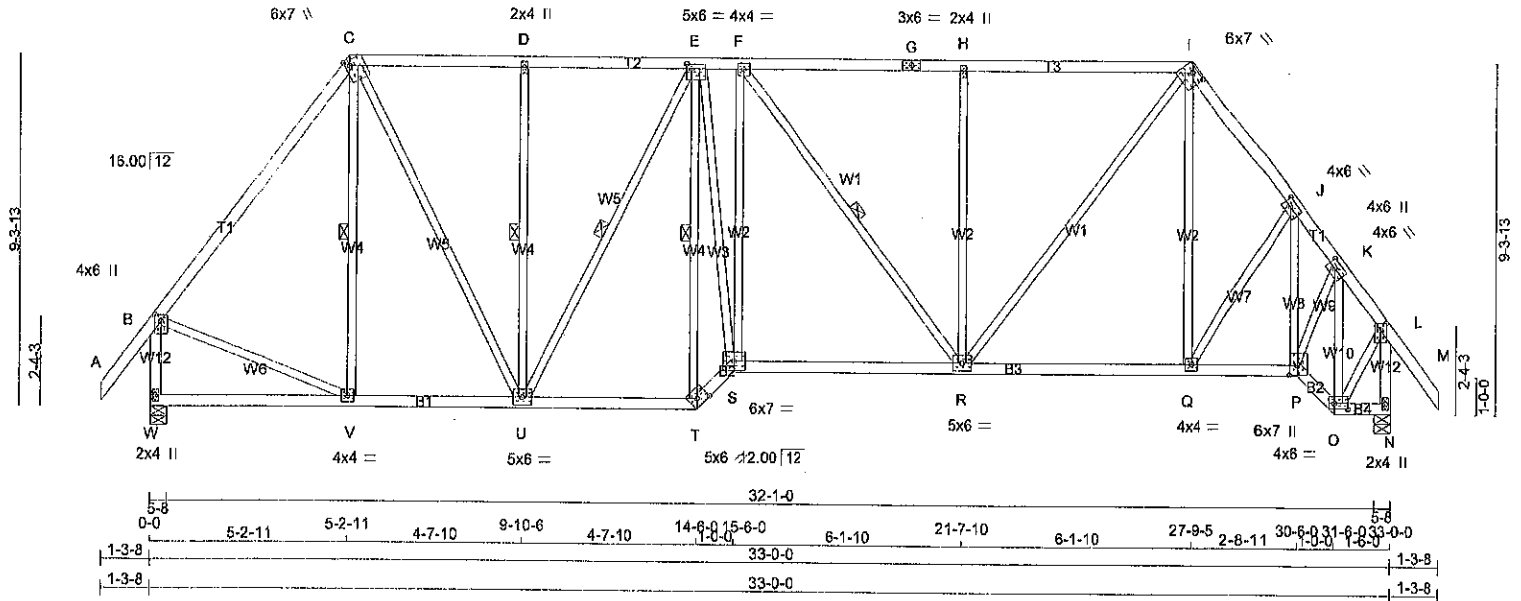


A-15073885(2)



A-1073884





LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - M	2x4	DRY	No.2	SPF
W - B	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
W - T	2x4	DRY	No.2	SPF
T - S	2x4	DRY	No.2	SPF
S - P	2x4	DRY	No.2	SPF
P - O	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TTWW+m	MT20	6.0	7.0	Edge	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMWWW-t	MT20	5.0	6.0	1.50	1.50
F	TMWW-t	MT20	4.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMW+w	MT20	2.0	4.0		
I	TTWW-h	MT20	6.0	7.0	1.50	3.75
J	TMWW-t	MT20	4.0	6.0	2.00	2.50
K	TMWW-t	MT20	4.0	6.0	2.00	2.50
L	TMVW+p	MT20	4.0	6.0	2.00	2.00
N	BMV1+p	MT20	2.0	4.0		
O	BBWW-t	MT20	4.0	6.0	1.75	4.25
P	BBWW+p	MT20	6.0	7.0	3.25	2.75
Q	BMWW-t	MT20	4.0	4.0		
R	BMWWW-t	MT20	5.0	6.0		
S	BBWW-t	MT20	6.0	7.0		
T	BBW-h	MT20	5.0	6.0	2.25	3.50
U	BMWWW-t	MT20	5.0	6.0		
V	BMWW-t	MT20	4.0	4.0		
W	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
W	1548	0	1548	0	5-6	1-11
N	1553	0	1553	0	5-6	1-11

UNFACTORED REACTIONS								
1ST LCASE		MAX/MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
W	1089	750 / 0	0 / 0	0 / 0	0 / 0	338 / 0	0 / 0	
N	1091	753 / 0	0 / 0	0 / 0	0 / 0	339 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, N

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.93FT.
 END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF C-V, D-U, E-U, E-T, F-R.
 END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
 TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 40	-70.5	-70.5 0.10 (1)	10.00	V-C	-161 / 25	0.09 (1)
B-C	-1202 / 0	-70.5	-70.5 0.45 (1)	5.36	C-U	0 / 897	0.20 (1)
C-D	-1128 / 0	-70.5	-70.5 0.23 (1)	5.70	U-D	-351 / 0	0.20 (1)
D-E	-1128 / 0	-70.5	-70.5 0.23 (1)	5.70	U-E	-423 / 0	0.31 (1)
E-F	-1499 / 0	-70.5	-70.5 0.16 (1)	5.17	T-E	-1249 / 0	0.71 (1)
F-G	-1386 / 0	-70.5	-70.5 0.48 (1)	4.93	E-S	0 / 1433	0.32 (1)
G-H	-1386 / 0	-70.5	-70.5 0.48 (1)	4.93	S-F	-57 / 31	0.08 (1)
H-I	-1386 / 0	-70.5	-70.5 0.48 (1)	4.93	F-R	-190 / 0	0.14 (1)
I-J	-1352 / 0	-70.5	-70.5 0.09 (1)	5.51	R-H	-498 / 0	0.68 (1)
J-K	-1255 / 0	-70.5	-70.5 0.06 (1)	5.88	R-I	0 / 963	0.22 (1)
K-L	-896 / 0	-70.5	-70.5 0.10 (1)	6.25	Q-I	0 / 78	0.03 (4)
L-M	0 / 40	-70.5	-70.5 0.10 (1)	10.00	Q-J	0 / 27	0.01 (1)
W-B	-1611 / 0	0.0	0.0 0.19 (1)	6.68	P-J	-263 / 0	0.09 (1)
N-L	-1639 / 0	0.0	0.0 0.20 (1)	6.63	P-K	0 / 824	0.19 (1)
					O-K	-1102 / 0	0.29 (1)
W-V	0 / 0	-17.5	-17.5 0.11 (4)	10.00	B-V	0 / 765	0.17 (1)
V-U	0 / 716	-17.5	-17.5 0.18 (1)	10.00	O-L	0 / 822	0.19 (1)
U-T	0 / 1318	-17.5	-17.5 0.26 (1)	10.00			
T-S	0 / 1843	-17.5	-17.5 0.30 (1)	10.00			
S-R	0 / 1500	-17.5	-17.5 0.33 (1)	10.00			
R-Q	0 / 798	-17.5	-17.5 0.23 (4)	10.00			
Q-P	0 / 783	-17.5	-17.5 0.18 (1)	10.00			
P-O	0 / 671	-17.5	-17.5 0.11 (1)	10.00			
O-N	0 / 0	-17.5	-17.5 0.01 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 21.0 PSF
 DL = 3.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

DESIGN ASSUMPTIONS
 - OVERHANG NOT TO BE ALTERED OR CUT OFF.

(56 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.10")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.09")
 ALLOWABLE DEFL.(TL) = L/360 (1.10")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.19")

CSI: TC=0.48 (H-I:1), BC=0.33 (R-S:1), WB=0.71 (E-T:1), SSI=0.21 (H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 822 2284 1856

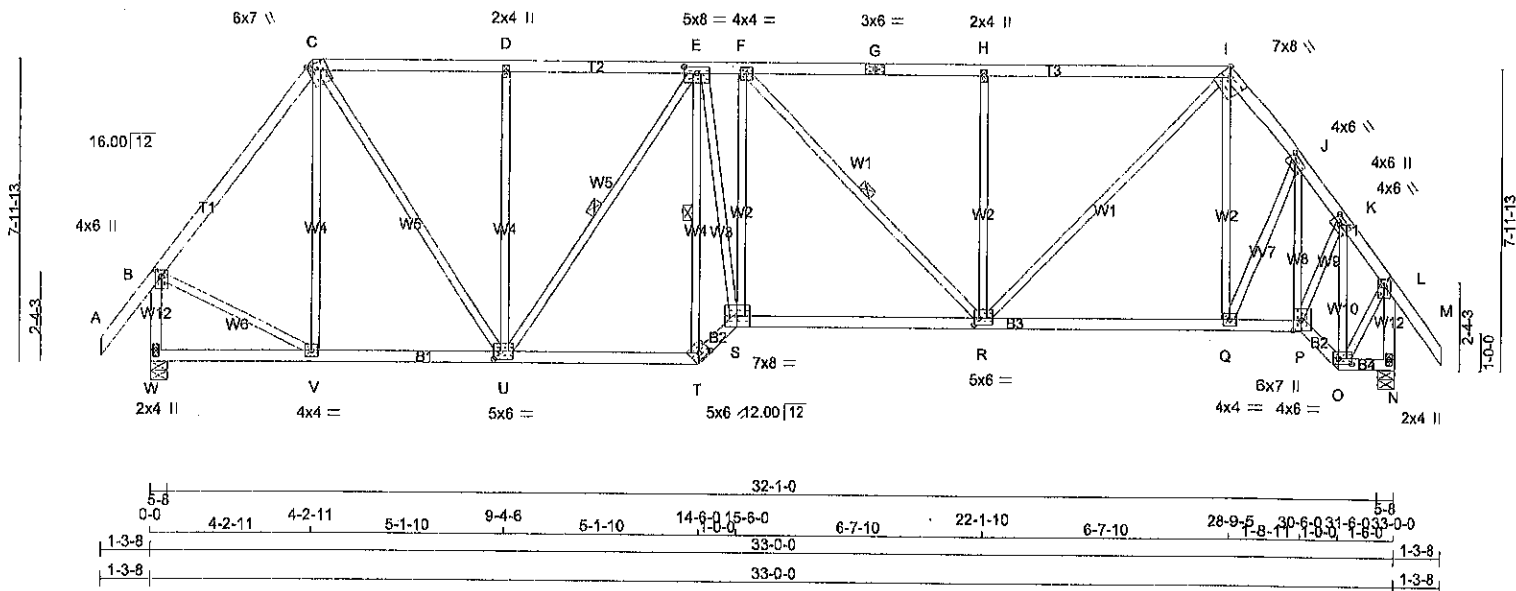
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (L) (INPUT = 0.90)
 JSI METAL= 0.38 (E) (INPUT = 1.00)



A-11073883



TOTAL WEIGHT = 179 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - M	2x4	DRY	No.2	SPF
W - B	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
W - T	2x4	DRY	No.2	SPF
T - S	2x4	DRY	No.2	SPF
S - P	2x4	DRY	No.2	SPF
P - O	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TTWW+m	MT20	6.0	7.0	Edge	1.50
D	TMVW+w	MT20	2.0	4.0		
E	TMVWW-I	MT20	5.0	8.0	2.00	4.00
F	TMVWW-I	MT20	4.0	4.0		
G	TS-I	MT20	3.0	6.0		
H	TMVW+w	MT20	2.0	4.0		
I	TTWW-h	MT20	7.0	8.0	Edge	2.75
J	TMVWW-I	MT20	4.0	6.0	2.00	2.60
K	TMVWW-I	MT20	4.0	6.0	2.00	2.60
L	TMVW+p	MT20	4.0	6.0	2.00	2.00
N	BMV1+p	MT20	2.0	4.0		
O	BBVW-I	MT20	4.0	6.0	1.75	4.25
P	BBVW+p	MT20	6.0	7.0	3.25	2.75
Q	BMVW-I	MT20	4.0	4.0		
R	BMVWW-I	MT20	5.0	6.0	2.25	1.50
S	BBVW-I	MT20	7.0	8.0		
T	BBVW-h	MT20	6.0	6.0	2.00	3.25
U	BMVWW-I	MT20	5.0	6.0	2.50	2.00
V	BMVW-I	MT20	4.0	4.0		
W	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
W	1548	0	1548	0
N	1553	0	1553	0

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
W	1089	750 / 0	0 / 0	0 / 0	338 / 0	0 / 0
N	1091	753 / 0	0 / 0	0 / 0	339 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.47FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF E-U, E-T, F-R.
 END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX (LC)	MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX (LC)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 40	-70.5	-70.5	0.10 (1)	10.00	V-C	-227 / 0
B-C	-1186 / 0	-70.5	-70.5	0.27 (1)	5.57	C-U	0 / 1052
C-D	-1291 / 0	-70.5	-70.5	0.30 (1)	5.33	U-D	-398 / 0
D-E	-1291 / 0	-70.5	-70.5	0.30 (1)	5.33	E-U	-470 / 0
E-F	-1803 / 0	-70.5	-70.5	0.21 (1)	4.77	T-E	-1472 / 0
F-G	-1635 / 0	-70.5	-70.5	0.69 (1)	4.47	E-S	0 / 1682
G-H	-1635 / 0	-70.5	-70.5	0.69 (1)	4.47	S-F	-68 / 32
H-I	-1634 / 0	-70.5	-70.5	0.69 (1)	4.47	F-R	-243 / 0
I-J	-1361 / 0	-70.5	-70.5	0.05 (1)	5.53	R-H	-536 / 0
J-K	-1268 / 0	-70.5	-70.5	0.05 (1)	5.68	H-I	0 / 1176
K-L	-895 / 0	-70.5	-70.5	0.10 (1)	6.25	Q-I	0 / 64
L-M	0 / 40	-70.5	-70.5	0.10 (1)	10.00	Q-J	0 / 72
W-B	-1520 / 0	0.0	0.0	0.19 (1)	6.67	P-J	-269 / 0
N-L	-1539 / 0	0.0	0.0	0.20 (1)	6.63	P-K	0 / 803
W-V	0 / 0	-17.5	-17.5	0.09 (4)	10.00	O-K	-1109 / 0
V-U	0 / 704	-17.5	-17.5	0.17 (1)	10.00	B-V	0 / 777
U-T	0 / 1548	-17.5	-17.5	0.31 (1)	10.00	O-L	0 / 825
T-S	0 / 2161	-17.5	-17.5	0.35 (1)	10.00		
S-R	0 / 1804	-17.5	-17.5	0.39 (1)	10.00		
R-Q	0 / 804	-17.5	-17.5	0.26 (4)	10.00		
Q-P	0 / 777	-17.5	-17.5	0.19 (1)	10.00		
P-O	0 / 873	-17.5	-17.5	0.11 (1)	10.00		
O-N	0 / 0	-17.5	-17.5	0.01 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	21.0	PSF
DL	=	3.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.0	PSF
TOTAL LOAD	=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/380 (1.10")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
 ALLOWABLE DEFL.(TL) = L/380 (1.10")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.23")

CSI: TC=0.59 (H-I:1), BC=0.39 (R-S:1), WB=0.69 (E-T:1), SSI=0.22 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	822	2284	1856

PLATE PLACEMENT TOL. = 0.250 inches

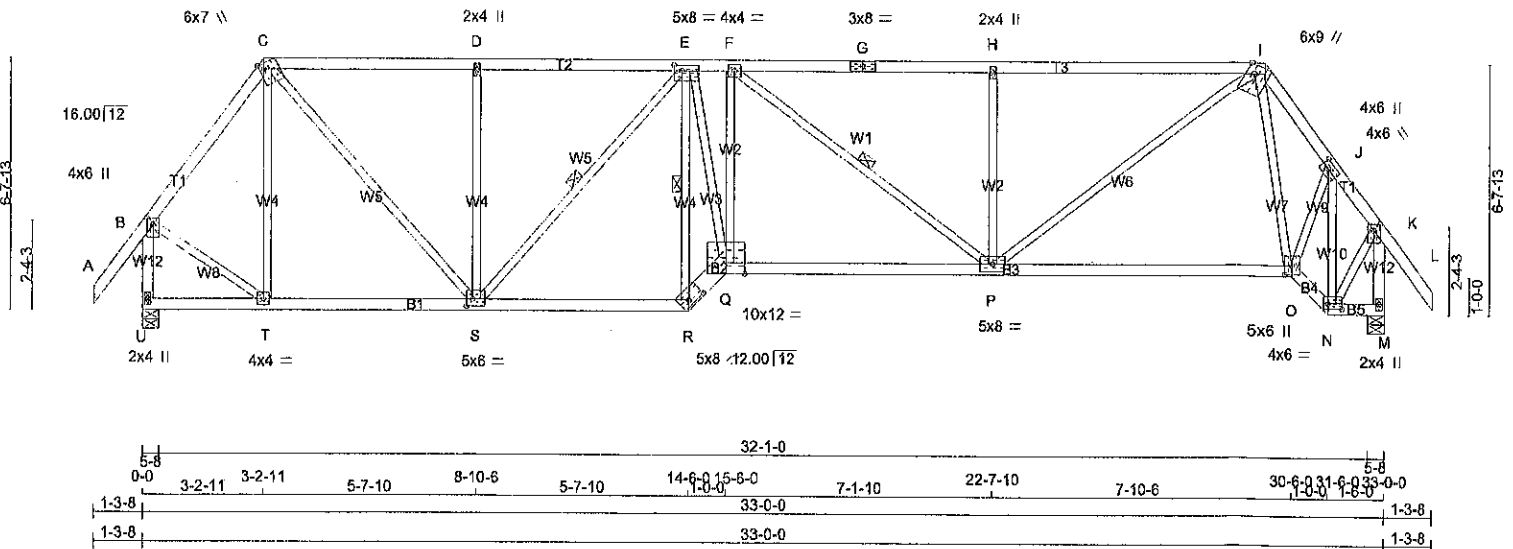
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (L) (INPUT = 0.90)
 JSI METAL= 0.37 (E) (INPUT = 1.00)



A-15073882

Alpa Roof Truss, Maple



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - L	2x4	DRY	No.2
U - B	2x4	DRY	No.2
M - K	2x4	DRY	No.2
U - R	2x4	DRY	No.2
R - Q	2x6	DRY	No.2
Q - O	2x4	DRY	No.2
O - N	2x4	DRY	No.2
N - M	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TTWW+m	MT20	8.0	7.0	Edge	1.50
D	TMVW+w	MT20	2.0	4.0		
E	TMVWW-I	MT20	5.0	6.0	1.75	2.50
F	TMVWW-I	MT20	4.0	4.0		
G	TS-t	MT20	3.0	8.0		
H	TMVW+w	MT20	2.0	4.0		
I	TTWW+m	MT20	6.0	9.0	Edge	2.00
J	TMVWW-I	MT20	4.0	6.0	2.00	2.50
K	BMVW+p	MT20	4.0	6.0	2.00	2.00
M	BMV1+p	MT20	2.0	4.0		
N	BBVW-I	MT20	4.0	6.0	1.75	4.50
O	BBVW+p	MT20	5.0	6.0	2.75	2.25
P	BMVWW-t	MT20	5.0	8.0		
Q	BBVW-I	MT20	10.0	12.0	Edge	
R	BBVW-h	MT20	5.0	8.0	1.75	5.00
S	BMVWW-t	MT20	6.0	6.0	2.25	2.00
T	BMVW-I	MT20	4.0	4.0		
U	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
JT	VERT	HORZ	FACTORED GROSS REACTION
U	1548	0	1548
M	1553	0	1553

UNFACTORED REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
U	1089	760 / 0	0 / 0	0 / 0	0 / 0	338 / 0
M	1091	763 / 0	0 / 0	0 / 0	0 / 0	339 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.83FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF E-S, E-R, F-P.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS			
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED HORIZ. LOAD (PLF)
A-B	0 / 40	-70.5	-70.5
B-C	-1142 / 0	-70.5	-70.5
C-D	-1519 / 0	-70.5	-70.5
D-E	-1519 / 0	-70.5	-70.5
E-F	-2285 / 0	-70.5	-70.5
F-G	-2009 / 0	-70.5	-70.5
G-H	-2009 / 0	-70.5	-70.5
H-I	-2009 / 0	-70.5	-70.5
I-J	-1276 / 0	-70.5	-70.5
J-K	-894 / 0	-70.5	-70.5
K-L	0 / 40	-70.5	-70.5
U-B	-1531 / 0	0.0	0.0
M-K	-1539 / 0	0.0	0.0
U-T	0 / 0	-17.5	-17.5
T-S	0 / 675	-17.5	-17.5
S-R	0 / 1883	-17.5	-17.5
R-Q	0 / 2636	-17.5	-17.5
Q-P	0 / 2286	-17.5	-17.5
P-O	0 / 781	-17.5	-17.5
O-N	0 / 677	-17.5	-17.5
N-M	0 / 0	-17.5	-17.5

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 21.0	PSF
	DL	= 3.0	PSF
BOT CH.	LL	= 0.0	PSF
	DL	= 7.0	PSF
TOTAL LOAD	=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.29")

CSI: TC=0.76 (H-I), BC=0.52 (P-Q), WB=0.51 (E-R), SSI=0.24 (H-I)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	818	354	1867
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (I) (INPUT = 0.90)
JSI METAL = 0.42 (E) (INPUT = 1.00)

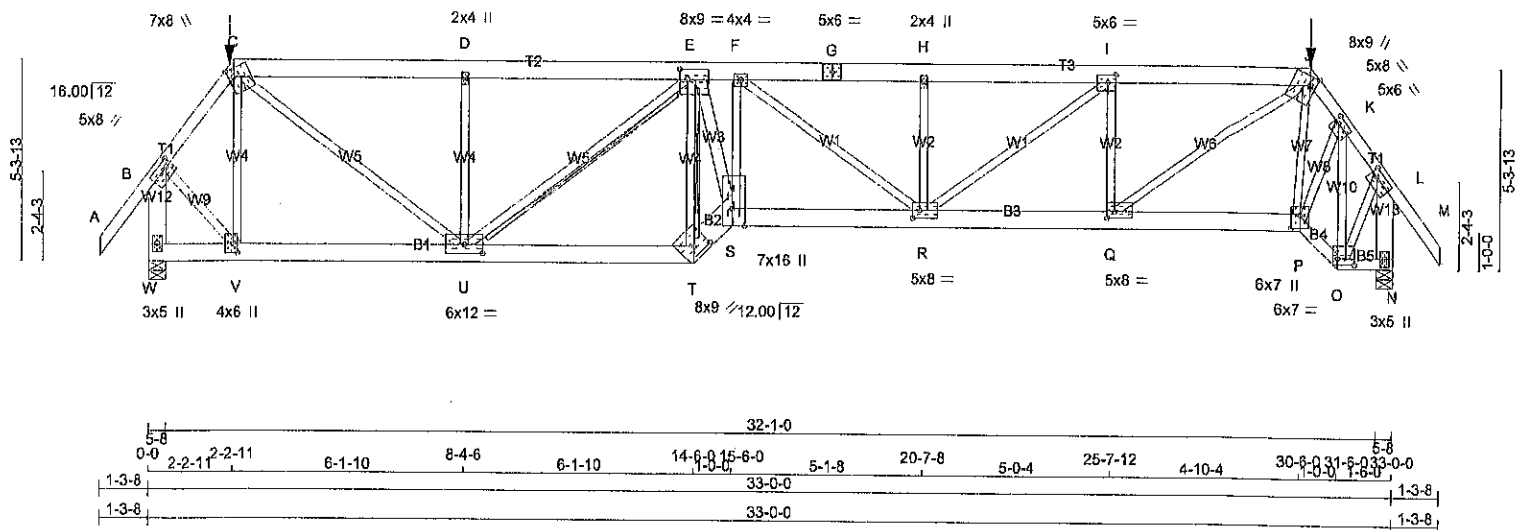


A-10073881

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253642	H01T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 5 Apr 16 2015 Mitek Industries, Inc. Mon Jul 20 08:14:10 2015 Page 1
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Scale = 1:61.4



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - G	2x4	DRY	No.2	SPF	
G - J	2x4	DRY	No.2	SPF	
J - M	2x4	DRY	No.2	SPF	
W - B	2x6	DRY	No.2	SPF	
N - L	2x6	DRY	No.2	SPF	
W - T	2x6	DRY	No.2	SPF	
T - S	2x8	DRY	No.2	SPF	
S - P	2x6	DRY	No.2	SPF	
P - O	2x4	DRY	No.2	SPF	
O - N	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
E - S	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (Table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMWV-I	MT20	5.0	8.0	2.00	2.00
C TTWW+m	MT20	7.0	8.0	2.50	1.75
D TMWV-I	MT20	2.0	4.0		
E TMWVW-I	MT20	8.0	9.0	3.25	2.25
F TMWV-I	MT20	4.0	4.0		
G TS-I	MT20	5.0	6.0		
H TMWV-I	MT20	2.0	4.0		
I TMWV-I	MT20	5.0	6.0	2.50	2.75
J TTWW+m	MT20	8.0	8.0	2.75	1.75
K TMWV-I	MT20	5.0	6.0	2.00	1.50
L TMWV-I	MT20	5.0	6.0	2.00	2.00
N BMV1+p	MT20	3.0	5.0		
O BBWV-I	MT20	8.0	7.0	2.00	5.50
P BBWV+p	MT20	6.0	7.0	4.00	3.00
Q BMWV-I	MT20	5.0	8.0	2.00	2.50
R BMWVW-I	MT20	5.0	8.0	2.50	2.25
S BBWV+p	MT20	7.0	18.0	Edge	4.00
T BBWV-I	MT20	8.0	9.0	2.75	4.50
U BMWVW-I	MT20	6.0	12.0	2.75	6.00
V BMWV-I	MT20	4.0	6.0	3.00	1.50
W BMV1+p	MT20	3.0	5.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
W	2919	0	2919	0
N	2907	0	2907	0

UNFACTORED REACTIONS

	1ST LCASE	MAX/MIN	COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
W	2055	1400 / 0	0 / 0	0 / 0	0 / 0	655 / 0	0 / 0	
N	2047	1393 / 0	0 / 0	0 / 0	0 / 0	654 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.13FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

2x4 DRY SPF No.2 T-BRACE REQUIRED AT E-U

2x4 DRY SPF No.2 I-BRACE REQUIRED AT E-T
FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LBS)	MEMB.	FORCE (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)
FR-TO				FR-TO			
A-B	0 / 40	-70.5	-70.5 0.11 (1)	10.00	V-C	-800 / 0	0.30 (1)
B-C	-2160 / 0	-70.5	-70.5 0.13 (1)	4.50	C-U	0 / 3202	0.79 (1)
C-D	-3788 / 0	-138.7	-138.7 0.49 (1)	3.91	U-D	-950 / 0	0.36 (1)
D-E	-3788 / 0	-138.7	-138.7 0.49 (1)	3.91	U-E	-1487 / 0	0.67 (1)
E-F	-6228 / 0	-138.7	-138.7 0.47 (1)	3.13	T-E	-4644 / 0	0.95 (1)
F-G	-5815 / 0	-70.5	-70.5 0.46 (1)	3.28	E-S	0 / 4909	0.87 (1)
G-H	-5815 / 0	-70.5	-70.5 0.46 (1)	3.28	S-F	0 / 503	0.12 (1)
H-I	-5815 / 0	-70.5	-70.5 0.34 (1)	3.42	F-R	-503 / 0	0.35 (1)
I-J	-4306 / 0	-70.5	-70.5 0.28 (1)	3.96	R-H	-423 / 0	0.11 (1)
J-K	-2591 / 0	-70.5	-70.5 0.14 (1)	4.16	R-I	0 / 1904	0.47 (1)
K-L	-1751 / 0	-70.5	-70.5 0.14 (1)	4.88	Q-I	-1522 / 0	0.39 (1)
L-M	0 / 40	-70.5	-70.5 0.11 (1)	10.00	P-J	-325 / 0	0.08 (1)
W-B	-2650 / 0	0.0	0.0 0.26 (1)	6.07	O-K	-2253 / 0	0.82 (1)
N-L	-2881 / 0	0.0	0.0 0.27 (1)	6.13	B-V	0 / 1862	0.41 (1)
W-V	0 / 0	-34.5	-34.5 0.12 (1)	10.00	O-L	0 / 1651	0.41 (1)
V-U	0 / 1277	-34.5	-34.5 0.29 (1)	10.00	P-K	0 / 1660	0.41 (1)
U-T	0 / 4947	-34.5	-34.5 0.76 (1)	10.00	Q-J	0 / 3450	0.85 (1)
T-S	0 / 6862	-34.5	-34.5 0.73 (1)	10.00			
S-R	0 / 6215	-102.7	-102.7 1.00 (1)	10.00			
R-Q	0 / 4306	-102.7	-102.7 0.69 (1)	10.00			
Q-P	0 / 1548	-102.7	-102.7 0.36 (1)	10.00			
P-O	0 / 1374	-34.5	-34.5 0.25 (1)	10.00			
O-N	0 / 0	-34.5	-34.5 0.02 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-2-11	-114	-114		FRONT	VERT	TOTAL
J	30-9-5	-114	-114		FRONT	VERT	TOTAL

TOTAL WEIGHT = 196 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	21.0	PSF
DL	=	3.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.0	PSF
TOTAL LOAD	=	31.0	PSF

SPACING = 24.0 IN. G/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip

SIDE SETBACK = 2-2-11
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDTL LOADS BASED ON 55% OF GSL.
LOADS APPLIED TO FIRST 16-6-0 OF SPAN MEASURED FROM THE LEFT.

GIRDER TYPE: CStdGirder

START DISTANCE = 15-6-0
START SPAN CARRIED = 5-10-8
END DISTANCE = 30-6-0
END SPAN CARRIED = 5-10-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDTL LOADS BASED ON 55% OF GSL.
(DEFINED BY USER)

GIRDER TYPE: CPrimeHip

SIDE SETBACK = 2-2-11
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDTL LOADS BASED ON 55% OF GSL.
LOADS APPLIED TO FIRST 2-8-0 OF SPAN MEASURED FROM THE RIGHT.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/998 (0.33")
ALLOWABLE DEFL.(TL) = L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/864 (0.60")

CSI: TC=0.49 (C-D:1), BC=1.00 (R-S:1),

AB=0.95 (T-S:1), BC=0.93 (S-T:1)
A-NOV-23-2020 CONTINUED ON PAGE 2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253642	H01T	1	1	TRUSS DESC.	

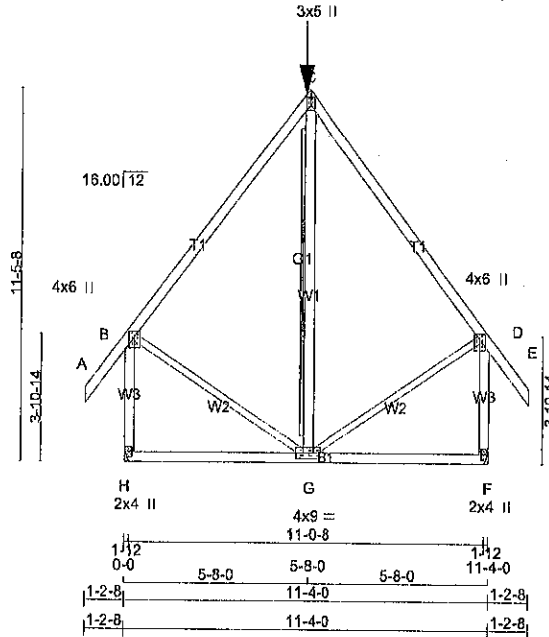
Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MITek Industries, Inc. Mon Jul 20 08:14:11 2015 Page 2
ID:AyrJx0Y?fa2EXF7DRQxSg4zMEz1-21Apf?1DaM70lMeUouxQRJf7dkEg2s8yM 8kxHywF?

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD. HANGERS NOTES 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 114.2 lbs FACTORED DOWN AT 2-2-11, AND 114.2 lbs FACTORED DOWN AT 30-9-5 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER. NOTES- (1) 1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.	DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00 COMPANION LIVE LOAD FACTOR = 0.50 AUTOSOLVE HEELS OFF TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES <table border="1"> <thead> <tr> <th>PLATE</th> <th>GRIP(DRY)</th> <th>SHEAR</th> <th>SECTION</th> </tr> <tr> <th>(PSI)</th> <th>(PLI)</th> <th>(PLI)</th> <th>(PLI)</th> </tr> </thead> <tbody> <tr> <td>MAX</td> <td>MIN</td> <td>MAX</td> <td>MIN</td> </tr> <tr> <td>MT20</td> <td>618</td> <td>354</td> <td>1667 822 2284 1656</td> </tr> </tbody> </table> PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.90 (Q) (INPUT = 0.90) JSI METAL= 0.78 (E) (INPUT = 1.00)	PLATE	GRIP(DRY)	SHEAR	SECTION	(PSI)	(PLI)	(PLI)	(PLI)	MAX	MIN	MAX	MIN	MT20	618	354	1667 822 2284 1656
PLATE	GRIP(DRY)	SHEAR	SECTION														
(PSI)	(PLI)	(PLI)	(PLI)														
MAX	MIN	MAX	MIN														
MT20	618	354	1667 822 2284 1656														



A-1507 3880(2)



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - E	2x4	DRY No.2	SPF
H - B	2x4	DRY No.2	SPF
F - D	2x4	DRY No.2	SPF
H - F	2x4	DRY No.2	SPF
ALL WEBS	2x4	DRY No.2	SPF
EXCEPT			
B - G	2x3	DRY No.2	SPF
G - D	2x3	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	6.0	2.00	2.00
C TTW+p	MT20	3.0	5.0	2.00	2.00
D TMVW+p	MT20	4.0	6.0	2.00	2.00
F BMV1+p	MT20	2.0	4.0		
G BMVWVH	MT20	4.0	9.0		
H BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 554.5 lbs FACTORED DOWN AT 5-8-0 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)
 1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
H	960	0	960	0		HANGER BY OTHERS
F	960	0	960	0		MIN. SEAT SIZE: 1-8 HANGER BY OTHERS MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN COMPONENT REACTIONS					SOIL
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	
H	876	459 / 0	0 / 0	0 / 0	0 / 0	218 / 0	0 / 0
F	876	459 / 0	0 / 0	0 / 0	0 / 0	218 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

2x4 DRY SPF No.2 T-BRACE REQUIRED AT C-G
 FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.
 END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		MAX. UNBRACED LENGTH	MEMB.	WEBS		MAX. UNBRACED LENGTH
	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	MAX			MAX. FACTORED FORCE (LBS)	MAX	
FR-TO						FR-TO			
A-B	0 / 37	-70.5	-70.5	0.10 (1)	10.00	G-C	-142 / 145	0.13 (1)	
B-C	-507 / 0	-70.5	-70.5	0.48 (1)	6.25	B-G	0 / 368	0.09 (1)	
C-D	-507 / 0	-70.5	-70.5	0.48 (1)	6.25	G-D	0 / 368	0.09 (1)	
D-E	0 / 37	-70.5	-70.5	0.10 (1)	10.00				
H-B	-888 / 0	0.0	0.0	0.24 (1)	7.81				
F-D	-888 / 0	0.0	0.0	0.24 (1)	7.81				
H-G	0 / 0	-33.5	-33.5	0.34 (4)	10.00				
G-F	0 / 0	-33.5	-33.5	0.34 (4)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	5-8-0	-554	-554	---	FRONT	VERT	TOTAL

TOTAL WEIGHT = 2 X 70 = 141 lb

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 21.0 PSF
 DL = 3.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CPrimeHp
 SIDE SETBACK = 5-8-0
 END SETBACK = 5-8-0
 END WALL WIDTH = 0-0
 CORNER FRAMING TYPE: CONVENTIONAL
 END JACK TYPE: CONVENTIONAL
 APPLIED TO FRONT SIDE
 - ADDTL LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
 8.4 P.S.F. RAIN LOAD EQUALS
 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.38")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
 ALLOWABLE DEFL.(TL) = L/360 (0.38")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.48 (C-D:1), BC=0.34 (G-H:4),
 WB=0.13 (C-G:1), SI=0.18 (F-G:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.60

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) SHEAR		SECTION	
	(PSI)	(PLI)	(PSI)	(PLI)
MT20	618	354	1667	822

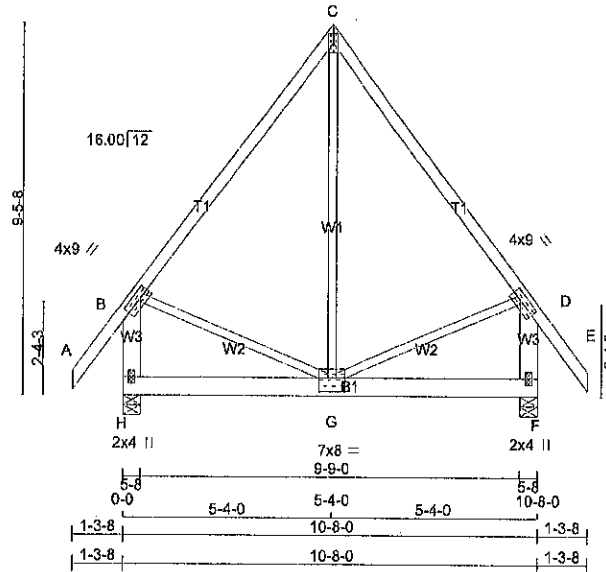
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 6.0 Deg.

JSI GRIP= 0.52 (B) (INPUT = 0.90)
 JSI METAL= 0.16 (F) (INPUT = 1.00)



A-1073879



LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x6	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
 EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	8.0	2.00	3.00
C	TTW+p	MT20	3.0	6.0		
D	TMVW-t	MT20	4.0	9.0	2.00	3.00
F	BMV1+p	MT20	2.0	4.0		
G	BMVWW-t	MT20	7.0	8.0	4.25	4.00
H	BMV1+p	MT20	2.0	4.0		

NOTES- (1)
 1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
H	1756	0	1756	0	5-8	1-14
F	1756	0	1756	0	5-8	1-14

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX/MIN. COMPONENT REACTIONS		PERM. LIVE WIND	DEAD	SOIL
	SNOW	LIVE	SNOW	LIVE			
H	1235	849 / 0	0 / 0	0 / 0	0 / 0	385 / 0	0 / 0
F	1235	849 / 0	0 / 0	0 / 0	0 / 0	385 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.71 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 40	-70.5 -70.5	0.11 (1)	G-C	0 / 1167	0.29 (1)	
B-C	-964 / 0	-70.5 -70.5	0.48 (1)	B-G	0 / 613	0.15 (1)	
C-D	-964 / 0	-70.5 -70.5	0.48 (1)	G-D	0 / 613	0.15 (1)	
D-E	0 / 40	-70.5 -70.5	0.11 (1)				
H-B	-1262 / 0	0.0 0.0	0.11 (1)				
F-D	-1262 / 0	0.0 0.0	0.11 (1)				
H-G	0 / 0	-240.1 -240.1	0.58 (1)				
G-F	0 / 0	-240.1 -240.1	0.58 (1)				

TOTAL WEIGHT = 68 lb (M)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 21.0 PSF
 DL = 3.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStd Girder
 START DISTANCE = 0-0
 START SPAN CARRIED = 12-1-8
 END DISTANCE = 10-8-0
 END SPAN CARRIED = 12-1-8
 END WALL WIDTH = 0-0
 APPLIED TO FRONT SIDE OF BOTTOM CHORD.
 - ADDTL LOADS BASED ON 66 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(65 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.36")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
 ALLOWABLE DEFL.(TL) = L/360 (0.36")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.08")

CSI: TC=0.48 (C-D:1), BC=0.58 (G-H:1), WB=0.29 (C-G:1), SSI=0.52 (F-G:1)

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 822 2284 1666

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.66 (C) (INPUT = 0.90)
 JSI METAL= 0.23 (H) (INPUT = 1.00)

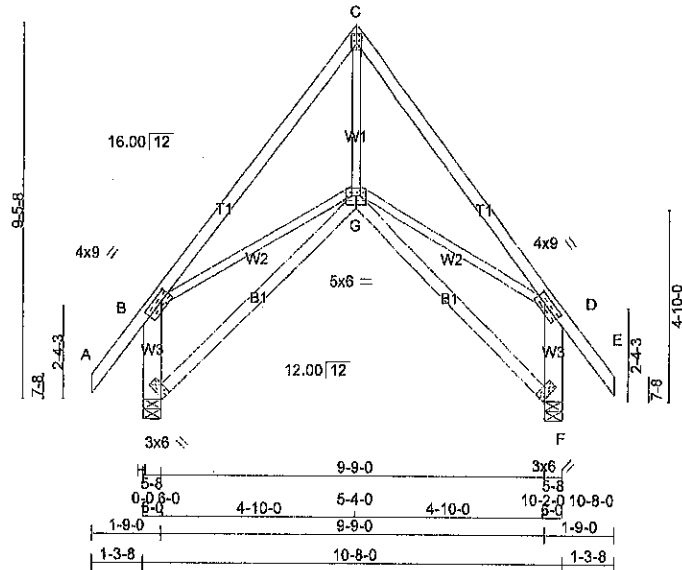


A-1073878

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253641	H13	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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ID:AyrJx9Y?fa2EXF7DRQxSq4zMEz1-MAeCA?hjjz8XYZ276NNakzCXdmFQ00krCuxRfywFWC
3x5 II
Scale = 1:58.3



TOTAL WEIGHT = 2 X 63 = 127 lb
[M][F]

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
H - B	2x6	DRY	No.2	SPF	
F - D	2x6	DRY	No.2	SPF	
H - G	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW-t	MT20	4.0	9.0	2.00	3.00
C	TTW+p	MT20	3.0	5.0	2.00	Edge
D	TMW-t	MT20	4.0	9.0	2.00	3.00
F	BVM1H	MT20	3.0	6.0	Edge	0.25
G	BBWWW-p	MT20	5.0	6.0	2.75	3.00
H	BVM1H	MT20	3.0	6.0	Edge	0.25

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		GROSS REACTION		MAXIMUM FACTORED		INPUT		REQRD	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	BRG	BRG	IN-SX	IN-SX
H	568	0	568	0	0	5-8	1-8				
F	568	0	568	0	0	5-8	1-8				

UNFACTORED REACTIONS		1ST LCASE		MAX / MIN. COMPONENT REACTIONS		LIVE		PERM. LIVE		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	SNOW	SNOW	SNOW	SNOW	SNOW	SNOW	SNOW	SNOW	SNOW	SNOW	SNOW	SNOW	SNOW
H	398	283 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0
F	398	283 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F
BEARING SIZE FACTOR = 1.15 AT JNT(S) H (BASED ON SUPPORT DEPTH = 1-8)
BEARING SIZE FACTOR = 1.15 AT JNT(S) F (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED		FACTORED		VERT. LOAD LC1		MAX		MAX		MEMB.		MAX. FACTORED		WEB S	
MEMB.	FORCE (LBS)	VERT.	PLF	CS1 (LC)	UNBRAC	LENGTH	FR-TO	MEMB.	FORCE (LBS)	VERT.	PLF	CS1 (LC)	UNBRAC	LENGTH	FR-TO	MEMB.	FORCE (LBS)
A-B	0 / 40	-70.5	-70.5	0.10 (1)	10.00	G-C	0 / 423	0.10 (1)									
B-C	-499 / 0	-70.5	-70.5	0.27 (1)	6.25	B-G	0 / 342	0.08 (1)									
C-D	-499 / 0	-70.5	-70.5	0.27 (1)	6.25	G-D	0 / 342	0.08 (1)									
D-E	0 / 40	-70.5	-70.5	0.10 (1)	10.00												
H-B	-522 / 0	0.0	0.0	0.05 (1)	7.81												
F-D	-522 / 0	0.0	0.0	0.05 (1)	7.81												
H-G	0 / 0	-17.5	-17.5	0.15 (4)	10.00												
G-F	0 / 0	-17.5	-17.5	0.15 (4)	10.00												

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 21.0 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.36")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.36")
CALCULATED VERT. DEFL.(TL) = L/999 (0.06")

CSI: TC=0.27 (C-D:1), BC=0.15 (F-G:4),
WB=0.10 (C-G:1), SSI=0.09 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1887 822 2284 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.66 (C) (INPUT = 0.90)
JSI METAL = 0.12 (B) (INPUT = 1.00)

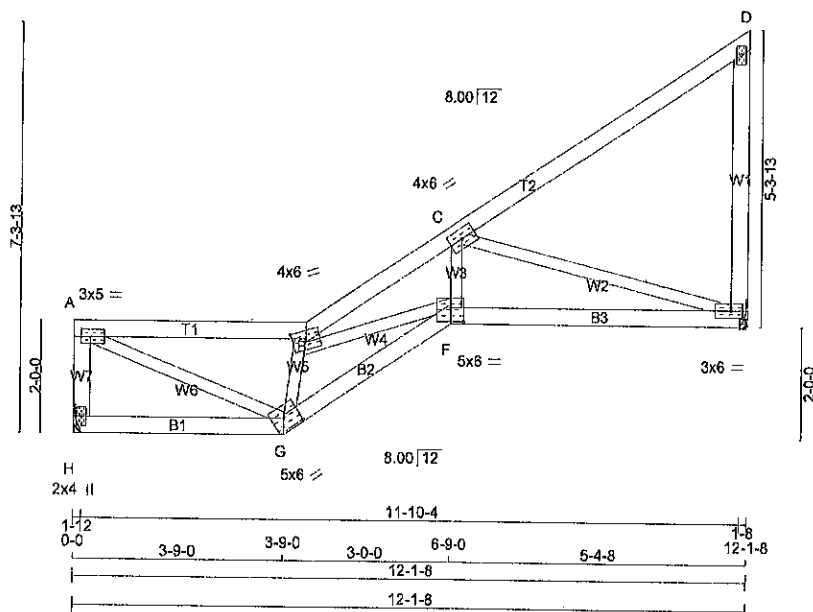


A-1073877

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253641	H12S	5	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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Scale = 1:41.3



LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
H - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-I	MT20	3.0	5.0	1.50	2.00
B	TTWW-m	MT20	4.0	6.0	1.75	3.00
C	TMWW-I	MT20	4.0	6.0		
D	TMV+p	MT20	2.0	4.0		
E	BMVW1-I	MT20	3.0	6.0	1.50	2.50
F	BBWW-I	MT20	5.0	6.0	3.00	3.00
G	BBWW-h	MT20	5.0	6.0	2.50	3.50
H	BMV1+p	MT20	2.0	4.0		

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT		REQD	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	533	0	533	0	0		
E	633	0	533	0	0		

HANGER BY OTHERS
MIN. SEAT SIZE: 1-8
HANGER BY OTHERS
MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	376	254 / 0	0 / 0	0 / 0	0 / 0	121 / 0	0 / 0
E	376	254 / 0	0 / 0	0 / 0	0 / 0	121 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MAX. FACTORED		FACTORED			MAX. FACTORED				
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	MAX UNBRAC	MEMB.	FORCE (LBS)	MAX	CSI (LC)	
FR-TO		FROM TO	LENGTH		FR-TO				
H-A	-500 / 0	0.0	0.0	0.06 (1)	7.81	A-G	0 / 868	0.20 (1)	
A-B	-791 / 0	-70.5	-70.5	0.16 (1)	6.25	G-B	-1007 / 0	0.15 (1)	
B-C	-1453 / 0	-70.5	-70.5	0.18 (1)	5.25	B-F	0 / 216	0.05 (1)	
C-D	-16 / 0	-70.5	-70.5	0.24 (1)	6.25	F-C	0 / 787	0.18 (1)	
E-D	-157 / 0	0.0	0.0	0.07 (1)	7.81	C-E	-1228 / 0	0.58 (1)	
H-G	0 / 0	-17.5	-17.5	0.07 (4)	10.00				
G-F	0 / 1230	-17.5	-17.5	0.22 (1)	10.00				
F-E	0 / 1182	-17.5	-17.5	0.28 (1)	10.00				

TOTAL WEIGHT = 5 X 49 = 246 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 21.0	PSF
	DL = 3.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.40")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.05")
ALLOWABLE DEFL.(TL) = $L/360$ (0.40")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.10")

CSI: TC=0.24 (C-D:1), BC=0.28 (E-F:1), WB=0.58 (C-E:1), SSI=0.14 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (E) (INPUT = 0.90)
JSI METAL= 0.37 (B) (INPUT = 1.00)

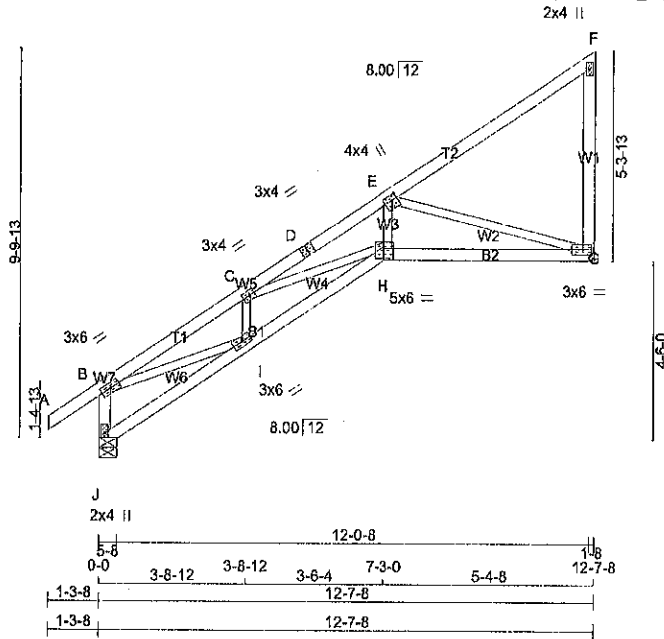


A-N073876

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253641	H12	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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Scale = 1:58.5



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
J - B	2x4	DRY	No.2	SPF	
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
H - G	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY:	SEASONED LUMBER.				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-I	MT20	3.0	8.0		
C	TMVW-I	MT20	3.0	4.0	1.50	1.50
D	TS-I	MT20	3.0	4.0		
E	TMVW-I	MT20	4.0	4.0	1.50	1.50
F	TMV+p	MT20	2.0	4.0		
G	BMVW-I	MT20	3.0	6.0	1.50	2.50
H	BMVW-I	MT20	5.0	6.0	3.00	3.00
I	BMVW-I	MT20	3.0	6.0		
J	BMV1+p	MT20	2.0	4.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 8 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
J	852	0	852	0
G	555	0	555	0

UNFACTORED REACTIONS

1ST LCASE	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	457	323 / 0	0 / 0	0 / 0	0 / 0	134 / 0	0 / 0
G	391	265 / 0	0 / 0	0 / 0	0 / 0	126 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.09FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	FORCE	FACTORED VERT. LOAD	LC1	MAX	MAX	MEMB.	FORCE	FACTORED VERT. LOAD	LC1	MAX
FR-TO		(LBS)									
J-B	-619 / 0	0.0	0.0	0.07 (1)	7.81	7.81	B-I	0 / 1133	0.26 (1)	0.26 (1)	0.26 (1)
A-B	0 / 27	-70.5	-70.5	0.15 (1)	10.00	10.00	I-C	-337 / 0	0.05 (1)	0.05 (1)	0.05 (1)
B-C	-1289 / 0	-70.5	-70.5	0.15 (1)	5.52	5.52	C-H	0 / 241	0.05 (1)	0.05 (1)	0.05 (1)
C-D	-1571 / 0	-70.5	-70.5	0.19 (1)	5.09	5.09	H-E	0 / 667	0.20 (1)	0.20 (1)	0.20 (1)
D-E	-1571 / 0	-70.5	-70.5	0.19 (1)	5.09	5.09	E-G	-1307 / 0	0.82 (1)	0.82 (1)	0.82 (1)
E-F	-16 / 0	-70.5	-70.5	0.24 (1)	6.25	6.25					
G-F	-158 / 0	0.0	0.0	0.08 (1)	7.81	7.81					
J-I	0 / 3	-17.5	-17.5	0.05 (4)	10.00	10.00					
I-H	0 / 1304	-17.5	-17.5	0.25 (1)	10.00	10.00					
H-G	0 / 1258	-17.5	-17.5	0.29 (1)	10.00	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	21.0	PSF
DL	=	3.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.0	PSF
TOTAL LOAD	=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(56 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.42")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.07")
ALLOWABLE DEFL.(TL) = $L/360$ (0.42")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.12")

CSI: TC=0.24 (E-F:1), BC=0.29 (G-H:1), WB=0.82 (E-G:1), SSI=0.14 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	618	354	1667
	822	2264	1656

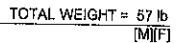
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

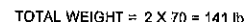
JSI GRIP= 0.88 (E) (INPUT = 0.90)
JSI METAL= 0.47 (E) (INPUT = 1.00)



A-11073875



A-15073874

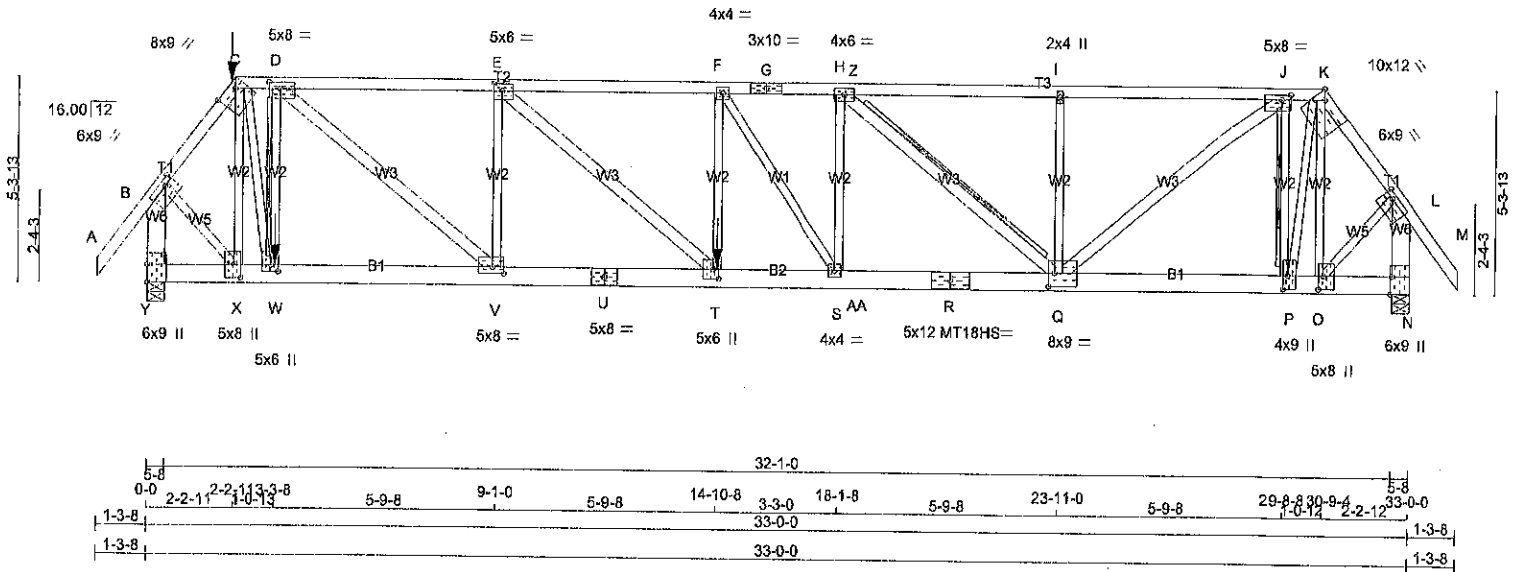


A-1073873

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253641	H01-Cond2	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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Scale = 1:80.3



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - G	2x4	DRY	2100F 1.8E
G - K	2x4	DRY	2100F 1.8E
K - M	2x4	DRY	No.2
Y - B	2x6	DRY	No.2
N - L	2x6	DRY	No.2
Y - U	2x6	DRY	1850F 1.5E
U - R	2x6	DRY	1850F 1.5E
R - N	2x6	DRY	1850F 1.5E
ALL WEBS EXCEPT	2x3	DRY	No.2
D - V	2x4	DRY	No.2
E - T	2x4	DRY	No.2
H - Q	2x4	DRY	No.2
O - J	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-I	MT20	6.0	9.0	1.75	3.00
C	TTWW-h	MT20	8.0	9.0	2.00	6.25
D	TMVW-I	MT20	5.0	8.0	2.00	3.50
E	TMVW-I	MT20	5.0	6.0	2.25	2.50
F	TMVW-I	MT20	4.0	4.0		
G	TS-I	MT20	3.0	10.0		
H	TMVW-I	MT20	4.0	8.0		
I	TMVW-h	MT20	2.0	4.0		
J	TMVW-I	MT20	5.0	8.0	1.50	3.00
K	TTWW-h	MT20	10.0	12.0	Edge	2.75
L	TMVW-I	MT20	6.0	9.0	1.50	2.75
N	BMV1-I	MT20	6.0	9.0	Edge	0.50
O	BMVW-I	MT20	5.0	8.0	4.25	1.50
P	BMVW-I	MT20	4.0	9.0	4.25	1.50
Q	BMVW-I	MT20	8.0	9.0	4.25	2.00
R	BS-I	MT18HS	5.0	12.0		
S	BMVW-I	MT20	4.0	4.0		
T	BMVW-I	MT20	5.0	6.0	3.00	1.75
U	BS-I	MT20	5.0	8.0		
V	BMVW-I	MT20	6.0	9.0	2.00	3.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
Y	4513	0	4513	0
N	4954	0	4954	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN	COMBINED	SNOW	LIVE	PERM	LIVE	WIND	DEAD	SOIL
Y	3178	2160 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	1018 / 0	0 / 0
N	3488	2376 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	1112 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Y, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.78 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.09 FT. OR RIGID CEILING DIRECTLY APPLIED.

2x4 DRY SPF No.2 T-BRACE REQUIRED AT D-W, H-Q, J-P
FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 8" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 80% OF WEB LENGTH.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 40	-70.5 -70.5 0.11 (1)	10.00	X-C	-552 / 0	0.22 (1)	
B-C	-3532 / 0	-70.5 -70.5 0.24 (1)	3.53	C-W	0 / 3108	0.77 (1)	
C-D	-2748 / 0	-138.7 -138.7 0.07 (1)	4.89	W-D	-2583 / 0	0.58 (1)	
D-E	-5592 / 0	-70.5 -70.5 0.53 (1)	3.38	D-V	0 / 3741	0.66 (1)	
E-F	-7649 / 0	-70.5 -70.5 0.76 (1)	2.76	V-E	-2217 / 0	0.90 (1)	
F-G	-7750 / 0	-138.8 -138.8 0.47 (1)	2.87	E-T	0 / 2704	0.48 (1)	
G-H	-7750 / 0	-138.8 -138.8 0.47 (1)	2.87	T-F	-553 / 0	0.22 (1)	
H-Z	-6521 / 0	-138.8 -138.8 0.61 (1)	3.11	F-S	0 / 184	0.05 (1)	
Z-I	-6521 / 0	-70.5 -70.5 0.61 (1)	3.11	S-H	0 / 598	0.15 (1)	
I-J	-6521 / 0	-70.5 -70.5 0.61 (1)	3.11	H-Q	-1618 / 0	0.56 (1)	
J-K	-2816 / 0	-70.5 -70.5 0.06 (1)	4.98	Q-I	-454 / 0	0.18 (1)	
K-L	-3768 / 0	-70.5 -70.5 0.27 (1)	3.41	Q-J	0 / 4720	0.83 (1)	
L-M	0 / 40	-70.5 -70.5 0.11 (1)	10.00	P-J	-3168 / 0	0.72 (1)	
Y-B	-4709 / 0	0.0 0.0 0.42 (1)	4.91	P-K	0 / 3300	0.82 (1)	
N-L	-5011 / 0	0.0 0.0 0.45 (1)	4.76	O-K	-139 / 0	0.18 (1)	
				B-X	0 / 2693	0.67 (1)	
				O-L	0 / 2872	0.71 (1)	
Y-X	0 / 0	-34.5 -34.5 0.26 (1)	10.00				
X-W	0 / 2088	-34.5 -34.5 0.46 (1)	10.00				
W-V	0 / 2747	-17.5 -17.5 0.36 (1)	10.00				
V-U	0 / 5592	-17.5 -17.5 0.55 (1)	10.00				
U-T	0 / 5592	-17.5 -17.5 0.55 (1)	10.00				
T-S	0 / 7649	-34.5 -34.5 0.76 (1)	10.00				
S-AA	0 / 7750	-34.5 -34.5 1.00 (1)	10.00				
AA-R	0 / 7750	-240.1 -240.1 1.00 (1)	10.00				
R-Q	0 / 7750	-240.1 -240.1 1.00 (1)	10.00				
Q-P	0 / 2939	-240.1 -240.1 0.61 (1)	10.00				
P-O	0 / 2228	-240.1 -240.1 0.58 (1)	10.00				
O-N	0 / 0	-240.1 -240.1 0.40 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE
C	2-2-11	-114	-114	---	FRONT	VERT	TOTAL
T	14-10-8	-1293	-1293	---	FRONT	VERT	TOTAL
W	3-3-8	-1293	-1293	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHlp
LEFT SETBACK = 2-2-11
RIGHT SETBACK = 2-2-12
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDTL. LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 3-3-8 OF SPAN MEASURED FROM THE LEFT.

GIRDER TYPE: CStdGlder
START DISTANCE = 18-6-0
START SPAN CARRIED = 12-1-8
END DISTANCE = 33-0-0
END SPAN CARRIED = 12-1-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDTL. LOADS BASED ON 55 % OF GSL.
(DEFINED BY USER)

*** NON STANDARD GIRDER ***
ADDTL. USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/969 (0.33")
ALLOWABLE DEFL.(TL) = L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/671 (0.59")

CSI: TC=0.78 (E-F:1), BC=1.00 (Q-S:1), W5=0.90 (E-V:1), SSI=0.57 (W-X:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253641	H01-Cond2	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
W	BMWWH	MT20	5.0	6.0	2.00	2.25
X	BMWWH	MT20	5.0	8.0	4.00	2.00
Y	BMV1H	MT20	6.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 114.2 lbs FACTORED DOWN AT 2-2-11 ON TOP CHORD, AND 1293.0 lbs FACTORED DOWN AT 3-3-8, AND 1293.0 lbs FACTORED DOWN AT 14-10-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

- 1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656
MT18HS	580	354	2729	1405	4191	2010

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

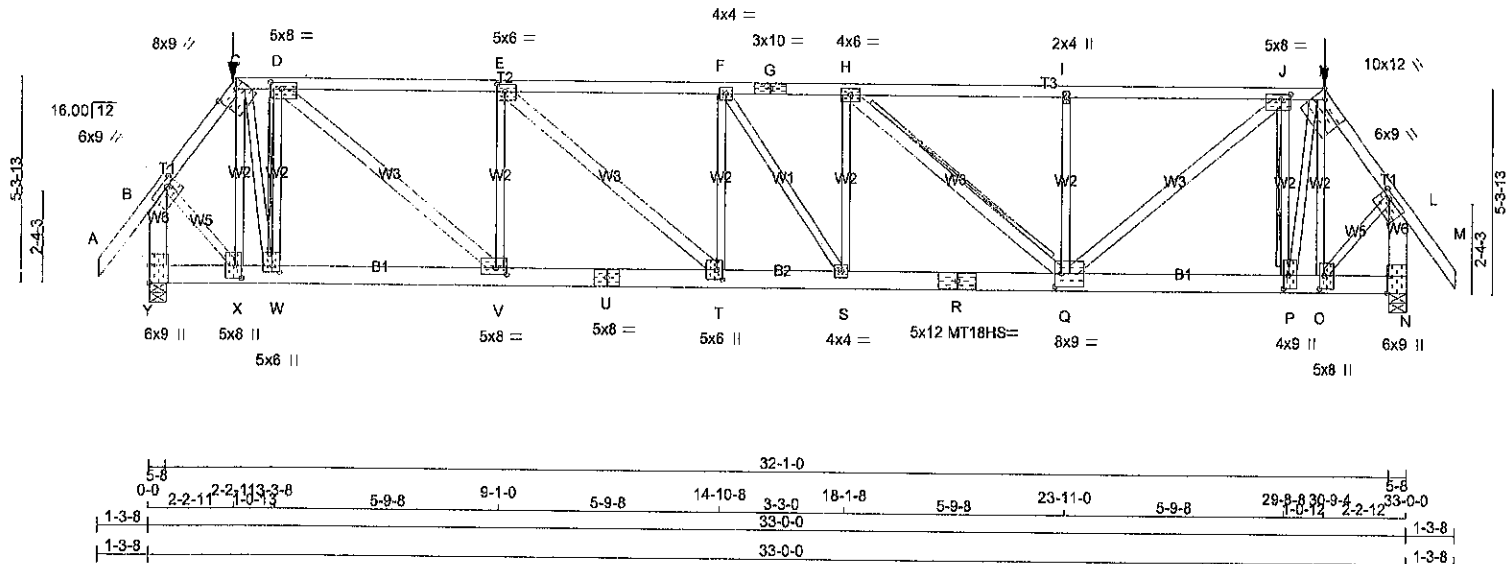
JSI GRIP = 0.88 (W) (INPUT = 0.80)

JSI METAL = 0.92 (U) (INPUT = 1.00)



A-11073872(2)

Alpa Roof Truss, Maple



LUMBER				N. L. G. A. RULES	
CHORDS	SIZE	LUMBER	DESCR.	SPF	
A - C	2x4	DRY	No.2	SPF	
C - G	2x4	DRY	2100F 1.8E	SPF	
G - K	2x4	DRY	2100F 1.8E	SPF	
K - M	2x4	DRY	No.2	SPF	
Y - B	2x6	DRY	No.2	SPF	
N - L	2x6	DRY	No.2	SPF	
Y - U	2x6	DRY	1650F 1.5E	SPF	
U - R	2x6	DRY	1650F 1.5E	SPF	
R - N	2x6	DRY	1650F 1.5E	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	
D - V	2x4	DRY	No.2	SPF	
E - T	2x4	DRY	No.2	SPF	
H - Q	2x4	DRY	No.2	SPF	
Q - J	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-I	MT20	6.0	9.0	1.75	3.00
C	TTWW-I	MT20	8.0	9.0	2.00	6.25
D	TMVW-I	MT20	6.0	8.0	2.00	3.50
E	TMVW-I	MT20	6.0	6.0	2.25	2.50
F	TMVW-I	MT20	4.0	4.0		
G	TS-I	MT20	3.0	10.0		
H	TMVW-I	MT20	4.0	6.0		
I	TMVW-I	MT20	2.0	4.0		
J	TMVW-I	MT20	5.0	8.0	1.50	3.00
K	TTWW-I	MT20	10.0	12.0	Edge	2.75
L	TMVW-I	MT20	8.0	9.0	1.50	2.75
N	BMV1-I	MT20	6.0	9.0	Edge	0.50
O	BMVW-I	MT20	5.0	8.0	4.25	1.50
P	BMVW-I	MT20	4.0	9.0	4.25	1.50
Q	BMVW-I	MT20	8.0	9.0	4.25	2.00
R	BS-I	MT18HS	5.0	12.0		
S	BMVW-I	MT20	4.0	4.0		
T	BMVW-I	MT20	5.0	6.0	3.00	1.75
U	BS-I	MT20	5.0	8.0		
V	BMVW-I	MT20	5.0	8.0	2.00	3.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	HORIZ	GROSS REACTION	HORIZ	BRG	IN-SX	BRG	IN-SX
Y	2919	0	0	2919	0	5-8	4-0	5-8	4-0
N	2919	0	0	2919	0	5-8	4-0	5-8	4-0

UNFACTORED REACTIONS

JT	1ST CASE	MAX	MIN	COMPONENT REACTIONS					
Y	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
Y	2055	1400 / 0	0 / 0	0 / 0	0 / 0	655 / 0	0 / 0		
N	2055	1400 / 0	0 / 0	0 / 0	0 / 0	655 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Y, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.33FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

2x4 DRY SPF No.2 T-BRACE REQUIRED AT D-W, H-Q, J-P
FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3"
COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	MEMB.	FORCE (LBS)	MAX UNBRAC LENGTH	MAX (LC)
FR-TO		FROM	TO	FR-TO			
A-B	0 / 40	-70.5	-70.5 0.11 (1)	X-C	-381 / 0	10.00	0.16 (1)
B-C	-2230 / 0	-70.5	-70.5 0.14 (1)	C-W	0 / 2075	4.44	0.51 (1)
C-D	-1750 / 0	-138.7	-138.7 0.23 (1)	W-D	-2409 / 0	5.71	0.54 (1)
D-E	-3930 / 0	-138.7	-138.7 0.64 (1)	D-V	0 / 2856	3.70	0.51 (1)
E-F	-4784 / 0	-138.7	-138.7 0.72 (1)	V-E	-1011 / 0	3.33	0.65 (1)
F-G	-4787 / 0	-138.7	-138.7 0.37 (1)	E-T	0 / 1123	3.67	0.20 (1)
G-H	-4787 / 0	-138.7	-138.7 0.37 (1)	T-F	-589 / 0	3.67	0.24 (1)
H-I	-3926 / 0	-138.7	-138.7 0.63 (1)	F-S	0 / 5	3.72	0.00 (1)
I-J	-3926 / 0	-138.7	-138.7 0.63 (1)	S-H	0 / 181	3.72	0.08 (4)
J-K	-1658 / 0	-138.7	-138.7 0.23 (1)	H-Q	-1132 / 0	5.82	0.40 (1)
K-L	-2243 / 0	-70.5	-70.5 0.14 (1)	Q-I	-878 / 0	4.43	0.36 (1)
L-M	0 / 40	-70.5	-70.5 0.11 (1)	Q-J	0 / 2853	10.00	0.50 (1)
Y-B	-3038 / 0	0.0	0.0 0.27 (1)	P-J	-2419 / 0	6.00	0.65 (1)
N-L	-3055 / 0	0.0	0.0 0.27 (1)	P-K	0 / 1911	5.99	0.47 (1)
Y-X	0 / 0	-34.5	-34.5 0.19 (1)	O-K	-215 / 0	10.00	0.09 (1)
X-W	0 / 1316	-34.5	-34.5 0.30 (1)	B-X	0 / 1700	10.00	0.42 (1)
W-V	0 / 1757	-34.5	-34.5 0.22 (1)	O-L	0 / 1710	10.00	0.42 (1)
V-U	0 / 3930	-34.5	-34.5 0.38 (1)				
U-T	0 / 3930	-34.5	-34.5 0.38 (1)				
T-S	0 / 4784	-34.5	-34.5 0.42 (1)				
S-R	0 / 4787	-34.5	-34.5 0.45 (1)				
R-Q	0 / 4787	-34.5	-34.5 0.45 (1)				
Q-P	0 / 1756	-34.5	-34.5 0.23 (1)				
P-O	0 / 1327	-34.5	-34.5 0.32 (1)				
O-N	0 / 0	-34.5	-34.5 0.21 (1)				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX	FACE	DIR.	TYPE
C	2-2-11	-114	-114	---	FRONT	VERT
K	30-9-4	-115	-115	---	FRONT	VERT

TOTAL WEIGHT = 192 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD	=	31.0	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHlp
LEFT SETBACK = 2-2-11
RIGHT SETBACK = 2-2-12
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADD'L LOADS BASED ON 66 % OF GSL.
LOADS APPLIED TO FIRST 33-0-0 OF SPAN MEASURED FROM THE LEFT.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(65 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/999 (0.20")
ALLOWABLE DEFL.(TL) = L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/999 (0.36")

CSI: TC=0.72 (E-F:1), BC=0.45 (G-S:1), WB=0.65 (E-V:1), SSI=0.43 (O-P:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1057 822 2284 1656
MT18HS 580 354 2728 1405 4191 2010

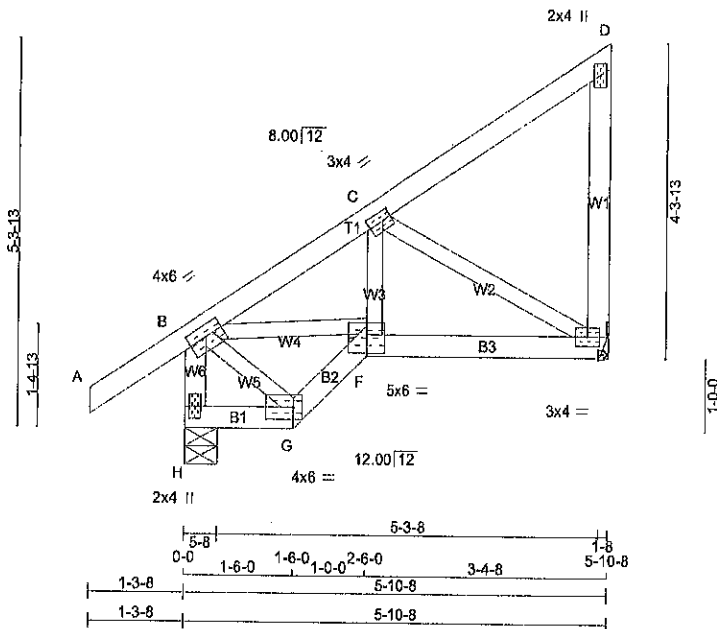
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

A-1073871



Alpa Roof Truss, Maple



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
H - B	2x4	DRY	No.2
A - D	2x4	DRY	No.2
E - D	2x4	DRY	No.2
H - G	2x4	DRY	No.2
G - F	2x4	DRY	No.2
F - E	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2

SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVWW-4	MT20	4.0	6.0	1.75	3.00
C	TMVWW-4	MT20	3.0	4.0	1.50	1.50
D	TMV-p	MT20	2.0	4.0		
E	BMVW1-t	MT20	3.0	4.0		
F	BBWW-4	MT20	6.0	6.0	3.00	3.00
G	BBW-4	MT20	4.0	6.0	2.00	4.50
H	BMV1+p	MT20	2.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
H	355	0	355	0
E	258	0	258	0

MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
H	248	181 / 0	0 / 0	0 / 0	0 / 0	0 / 0	87 / 0	0 / 0
E	182	123 / 0	0 / 0	0 / 0	0 / 0	0 / 0	59 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)	
H-B	-342 / 0	0.0	0.0 (1)	H-G	0 / 18	0.01 (4)	
A-B	0 / 27	-70.5	-70.5 (1)	B-G	0 / 238	0.05 (1)	
B-C	-287 / 0	-70.5	-70.5 (1)	F-C	0 / 57	0.02 (4)	
C-D	-12 / 0	-70.5	-70.5 (1)	C-E	-284 / 0	0.07 (1)	
E-D	-93 / 0	0.0	0.0 (1)				
H-G	0 / 0	-17.5	-17.5 (4)				
G-F	0 / 20	-17.5	-17.5 (4)				
F-E	0 / 248	-17.5	-17.5 (4)				

TOTAL WEIGHT = 8 X 30 = 238 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/898 (0.01")

CSI: TC=0.10 (B-C:1), BC=0.08 (E-F:4), WB=0.07 (C-E:1), SSI=0.09 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.60

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1867 822 2284 1656

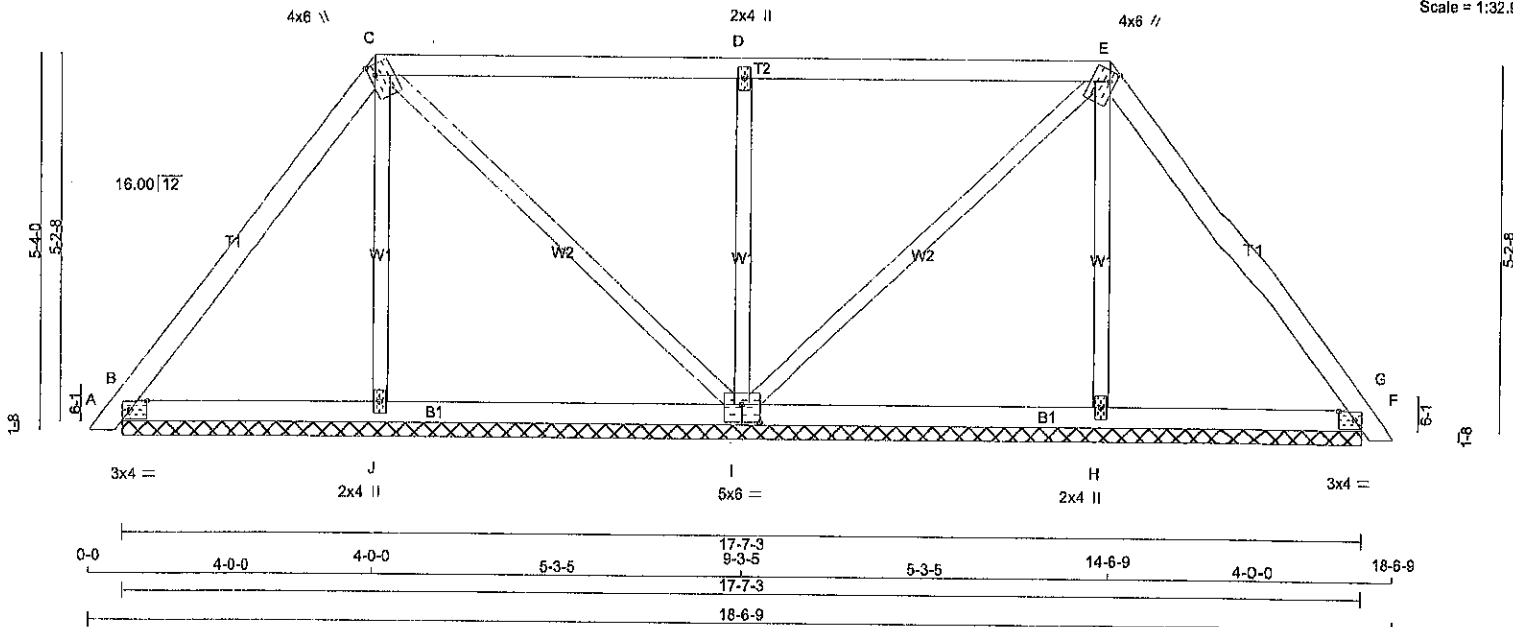
PLATE PLACEMENT TOL. = 0.260 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.41 (B) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)



A-12073861



TOTAL WEIGHT = 3 X 70 = 209 lb

LUMBER				
N. L. G. A. RULES	SIZE	DRY	No.2	DESCR.
CHORDS				
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
B - I	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	X
B	TMB1-I	MT20	3.0	4.0	1.50 2.75
C	TTWW+m	MT20	4.0	6.0	1.75 1.00
D	TTWW+m	MT20	2.0	4.0	
E	TTWW+m	MT20	4.0	6.0	1.75 1.00
F	TMB1-I	MT20	3.0	4.0	1.50 2.75
H	BMW1+w	MT20	2.0	4.0	
I	BSWWV1-I	MT20	5.0	6.0	3.00 3.00
J	BMW1+w	MT20	2.0	4.0	

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.11.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT		VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
B	245	0	245	0	0	0	17-7-3 (8-9-10)8		
J	231	0	231	0	0	0	17-7-3 (8-9-10)8		
I	646	0	646	0	0	0	17-7-3 (8-9-10)8		
H	231	0	231	0	0	0	17-7-3 (8-9-10)8		
F	245	0	245	0	0	0	17-7-3 (8-9-10)8		

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
B	169	132 / 0	0 / 0	0 / 0	0 / 0	37 / 0	0 / 0	
J	167	87 / 0	0 / 0	0 / 0	0 / 0	80 / 0	0 / 0	
I	451	329 / 0	0 / 0	0 / 0	0 / 0	122 / 0	0 / 0	
H	187	87 / 0	0 / 0	0 / 0	0 / 0	80 / 0	0 / 0	
F	169	132 / 0	0 / 0	0 / 0	0 / 0	37 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, J, I, H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 MAX	MEMB.	MAX. FACTORED FORCE (LBS)	MAX LC1 MAX	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 9	-70.5 -70.5	0.01 (1)	10.00	J-C	-146 / 0	0.06 (1)
B-C	-91 / 0	-70.5 -70.5	0.15 (1)	6.25	C-I	-88 / 0	0.06 (1)
C-D	-1 / 0	-70.5 -70.5	0.33 (1)	10.00	I-D	-484 / 0	0.19 (1)
D-E	-1 / 0	-70.5 -70.5	0.33 (1)	10.00	I-E	-66 / 0	0.06 (1)
E-F	-91 / 0	-70.5 -70.5	0.15 (1)	6.25	H-E	-146 / 0	0.06 (1)
F-G	0 / 8	-70.5 -70.5	0.01 (1)	10.00			
B-J	0 / 53	-17.5 -17.5	0.08 (4)	10.00			
J-I	0 / 50	-17.5 -17.5	0.10 (4)	10.00			
I-H	0 / 50	-17.5 -17.5	0.10 (4)	10.00			
H-F	0 / 53	-17.5 -17.5	0.08 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 21.0 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(65 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.33 (C-D:1), BC=0.10 (H-I:4), WB=0.19 (D-I:1), SSI=0.18 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.60

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1867 822 2284 1656

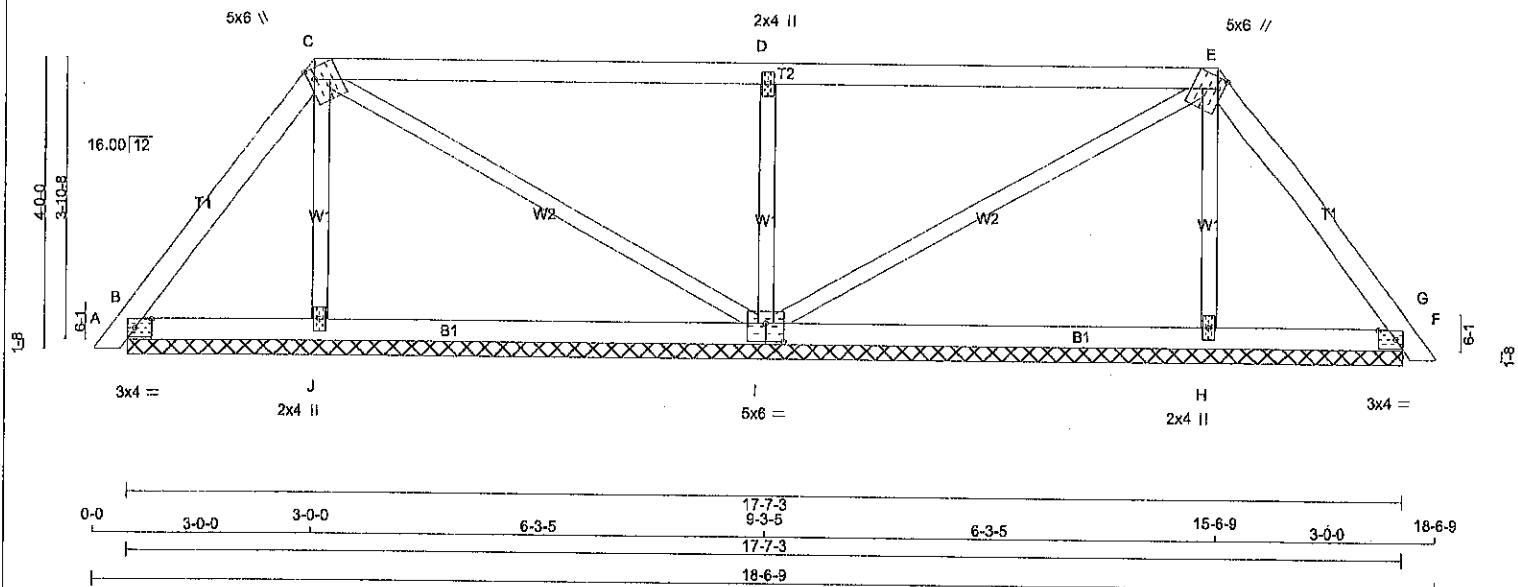
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (F) (INPUT = 0.90)
JSI METAL= 0.21 (I) (INPUT = 1.00)



A-15073852



LUMBER
 N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2 SPF
C - E	2x4	DRY	No.2 SPF
E - G	2x4	DRY	No.2 SPF
B - I	2x4	DRY	No.2 SPF
I - F	2x4	DRY	No.2 SPF
ALL WEBS	2x3	DRY	No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.75
C	TTWW+m	MT20	5.0	6.0	1.75	1.00
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	1.75	1.00
F	TMB1-I	MT20	3.0	4.0	1.50	2.75
H	BMW1+w	MT20	2.0	4.0		
I	BSWWW1-I	MT20	5.0	6.0	3.00	3.00
J	BMW1+w	MT20	2.0	4.0		

NOTES: (1)
 1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
B	193	0	193	0	17-7-3 (8-9-10) 8	17-7-3 (8-9-10) 8
J	252	0	252	0	17-7-3 (8-9-10) 8	17-7-3 (8-9-10) 8
I	709	0	709	0	17-7-3 (8-9-10) 8	17-7-3 (8-9-10) 8
H	252	0	252	0	17-7-3 (8-9-10) 8	17-7-3 (8-9-10) 8
F	193	0	193	0	17-7-3 (8-9-10) 8	17-7-3 (8-9-10) 8

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	132	110 / 0	0 / 0	0 / 0	0 / 0	22 / 0	0 / 0
J	182	97 / 0	0 / 0	0 / 0	0 / 0	85 / 0	0 / 0
I	496	354 / 0	0 / 0	0 / 0	0 / 0	143 / 0	0 / 0
H	182	97 / 0	0 / 0	0 / 0	0 / 0	85 / 0	0 / 0
F	132	110 / 0	0 / 0	0 / 0	0 / 0	22 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, J, I, H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX CS1 (LC)	MAX UNBRAC LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CS1 (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 9	-70.5 -70.5	0.01 (1)	10.00	J-C	-163 / 0	0.04 (1)
B-C	-88 / 0	-70.5 -70.5	0.08 (1)	6.25	C-I	-42 / 0	0.04 (1)
C-D	-9 / 0	-70.5 -70.5	0.47 (1)	10.00	I-D	-552 / 0	0.13 (1)
D-E	-9 / 0	-70.5 -70.5	0.47 (1)	10.00	E-H	-42 / 0	0.04 (1)
E-F	-88 / 0	-70.5 -70.5	0.08 (1)	6.25	H-E	-163 / 0	0.04 (1)
F-G	0 / 9	-70.5 -70.5	0.01 (1)	10.00			
B-J	0 / 51	-17.5 -17.5	0.10 (4)	10.00			
J-I	0 / 46	-17.5 -17.5	0.15 (4)	10.00			
I-H	0 / 46	-17.5 -17.5	0.15 (4)	10.00			
H-F	0 / 51	-17.5 -17.5	0.10 (4)	10.00			

TOTAL WEIGHT = 2 X 65 = 130 lb

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 21.0 PSF
 DL = 3.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55 % OF 23.0 P.S.F. G.S.I. PLUS
 8.4 P.S.F. RAIN LOAD EQUALS
 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.47 (C-D:1), BC=0.15 (I-J:4), WB=0.13 (D-I:1), SSI=0.22 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP (DRY) SHEAR SECTION
 (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 364 1687 822 2284 1656

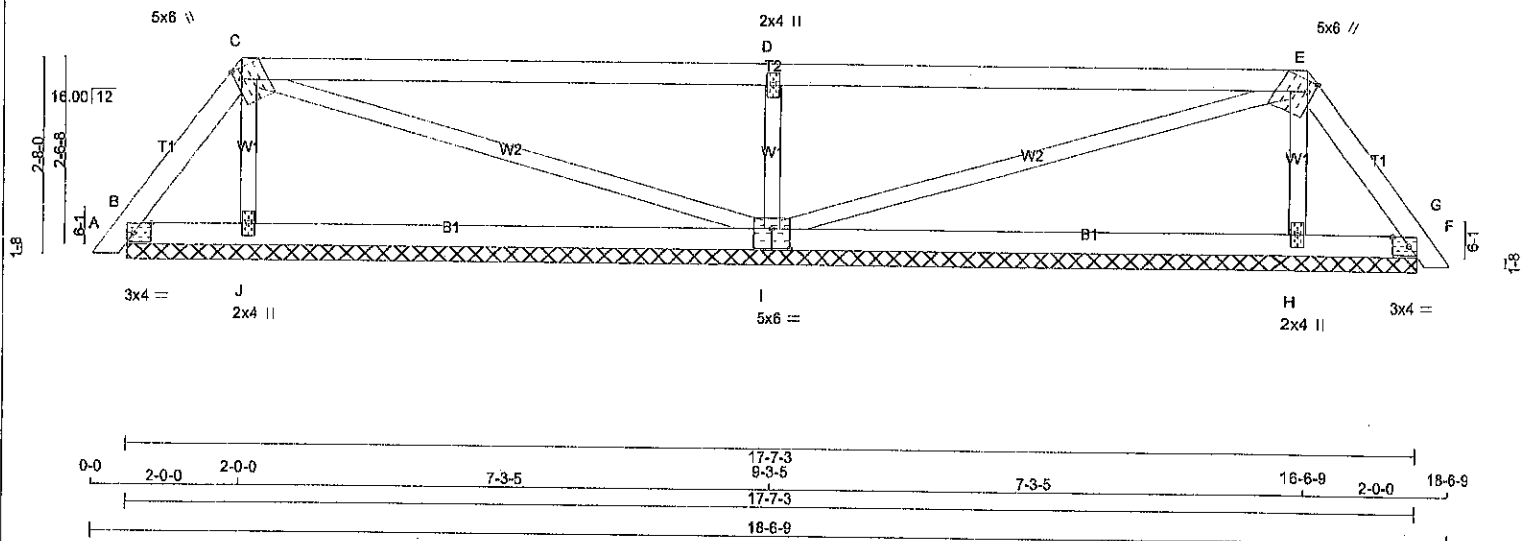
PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.80 (B) (INPUT = 0.80)
 JSI METAL = 0.23 (I) (INPUT = 1.00)



A-15073851



LUMBER
 N. L. G. A. RULES
 CHORDS SIZE LUMBER DESCR. SPF

A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
B - I	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.75
C	TTWW+m	MT20	5.0	6.0	1.75	1.00
D	TTWW+m	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	1.75	1.00
F	TMB1-I	MT20	3.0	4.0	1.50	2.75
H	BMW1+w	MT20	2.0	4.0		
I	BSWWV1-I	MT20	5.0	6.0	3.25	3.00
J	BMW1+w	MT20	2.0	4.0		

NOTES: (1)
 1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 8.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
B	119	0	119	0	-8	17-7-3 (8-9-10)8
J	287	0	287	0	0	17-7-3 (8-9-10)8
I	785	0	785	0	0	17-7-3 (8-9-10)8
H	287	0	287	0	0	17-7-3 (8-9-10)8
F	119	0	119	0	-8	17-7-3 (8-9-10)8

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 150 LBS FACTORED UPLIFT
 PROVIDE ANCHORAGE AT BEARING JOINT F FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS							
1ST LCASE		MAX/MIN COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
B	79	84 / 0	0 / 0	0 / 0	0 / 0	0 / -5	0 / 0
J	208	105 / 0	0 / 0	0 / 0	0 / 0	103 / 0	0 / 0
I	550	389 / 0	0 / 0	0 / 0	0 / 0	181 / 0	0 / 0
H	208	105 / 0	0 / 0	0 / 0	0 / 0	103 / 0	0 / 0
F	79	84 / 0	0 / 0	0 / 0	0 / 0	0 / -5	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, J, I, H, F

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING
 TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 9	-70.5 -70.5	0.01 (1)	10.00	J-C	-177 / 0	0.03 (1)
B-C	-76 / 0	-70.5 -70.5	0.03 (1)	6.25	C-I	-22 / 0	0.03 (1)
C-D	-13 / 0	-70.5 -70.5	0.63 (1)	6.25	I-D	-640 / 0	0.10 (1)
D-E	-13 / 0	-70.5 -70.5	0.63 (1)	6.25	I-E	-22 / 0	0.03 (1)
E-F	-76 / 0	-70.5 -70.5	0.03 (1)	6.25	H-E	-177 / 0	0.03 (1)
F-G	0 / 9	-70.5 -70.5	0.01 (1)	10.00			
B-J	0 / 43	-17.5 -17.5	0.14 (4)	10.00			
J-I	0 / 35	-17.5 -17.5	0.20 (4)	10.00			
I-H	0 / 35	-17.5 -17.5	0.20 (4)	10.00			
H-F	0 / 43	-17.5 -17.5	0.14 (4)	10.00			

TOTAL WEIGHT = 2 X 61 = 122 lb (M)

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 21.0 PSF
 DL = 3.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-06
 - TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
 8.4 P.S.F. RAIN LOAD EQUALS
 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.63 (D-E:1), BC=0.20 (I-J:4), WB=0.10 (D-I:1), SSI=0.25 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

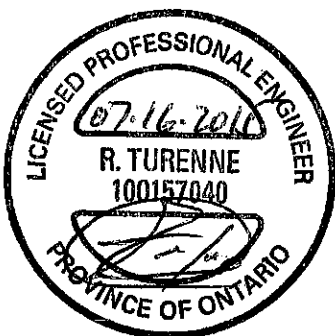
NAIL VALUES
 PLATE GRIP (DRY) SHEAR SECTION
 (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1687 822 2284 1656

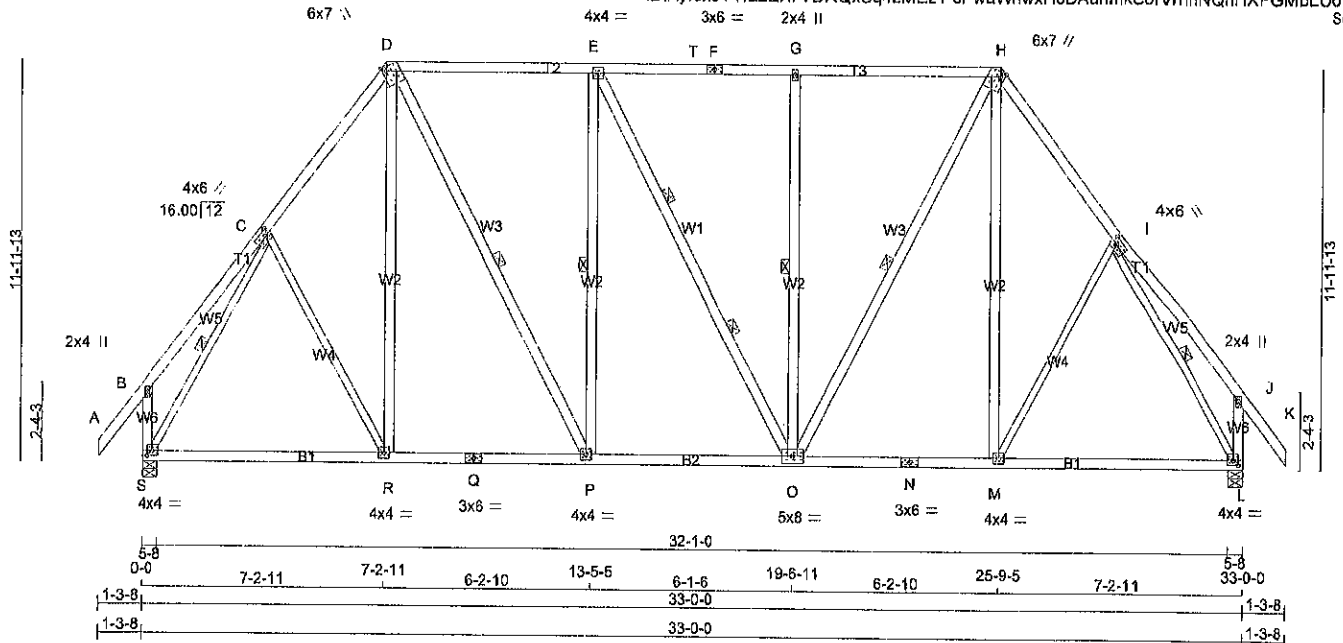
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (I) (INPUT = 0.90)
 JSI METAL= 0.24 (I) (INPUT = 1.00)

A-N073781





TOTAL WEIGHT = 9 X 216 = 1944 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
S - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
S - Q	2x4	DRY	No.2
Q - N	2x4	DRY	No.2
N - L	2x4	DRY	No.2
ALL WEBS	2x4	DRY	No.2
EXCEPT			
C - R	2x3	DRY	No.2
M - I	2x3	DRY	No.2
S - C	2x3	DRY	No.2
I - L	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWW-t	MT20	4.0	6.0	2.00	2.00
D	TTWW+m	MT20	6.0	7.0	Edge	1.50
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW+m	MT20	6.0	7.0	Edge	1.50
I	TMWW-t	MT20	4.0	6.0	2.00	2.00
J	TMV+p	MT20	2.0	4.0		
L	BMVW1-t	MT20	4.0	4.0	1.75	1.75
M, P, R						
M	BMWW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	6.0		
O	BMWWW-t	MT20	6.0	8.0		
Q	BS-t	MT20	3.0	6.0		
S	BMVW1-t	MT20	4.0	4.0	1.75	1.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
JT	VERT	GROSS REACTION	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
S	1620	0	1620	0	0	5-8	1-12		
L	1620	0	1620	0	0	5-8	1-12		

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN.	COMBINED	LIVE	PERM. LIVE	WIND	DEAD	SOIL
S	1146	752 / 0	0 / 0	0 / 0	0 / 0	394 / 0	0 / 0	0 / 0
L	1146	752 / 0	0 / 0	0 / 0	0 / 0	394 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.57FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF D-P, E-P, G-O, H-O, C-S, I-L.

2 LATERAL BRACE(S) REQUIRED AT 1/3 LENGTH OF E-O.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)	MAX. CSI (LC)	MAX. CSI (LC)
FR-TO		FROM	TO	FR-TO			
A-B	0 / 40	-70.5	-70.5 0.10 (1)	10.00	C-R	0 / 64	0.02 (4)
B-C	0 / 23	-70.5	-70.5 0.14 (1)	10.00	R-D	0 / 97	0.02 (4)
C-D	-1286 / 0	-70.5	-70.5 0.14 (1)	5.57	D-P	0 / 702	0.11 (1)
D-E	-1085 / 0	-78.0	-78.0 0.36 (1)	5.62	P-E	-519 / 0	0.41 (1)
E-T	-1085 / 0	-78.0	-78.0 0.33 (1)	5.66	E-O	-2 / 0	0.00 (1)
T-F	-1085 / 0	-78.0	-78.0 0.33 (1)	5.66	O-G	-518 / 0	0.41 (1)
F-G	-1085 / 0	-78.0	-78.0 0.33 (1)	5.66	G-H	0 / 700	0.11 (1)
G-H	-1084 / 0	-78.0	-78.0 0.36 (1)	5.62	H-I	0 / 98	0.02 (4)
H-I	-1286 / 0	-70.5	-70.5 0.14 (1)	5.57	I-L	0 / 64	0.02 (4)
I-J	0 / 23	-70.5	-70.5 0.14 (1)	10.00	S-C	-1552 / 0	0.60 (1)
J-K	0 / 40	-70.5	-70.5 0.10 (1)	10.00	L-L	-1553 / 0	0.60 (1)
S-B	-196 / 0	0.0	0.0 0.02 (1)	7.81			
L-J	-198 / 0	0.0	0.0 0.02 (1)	7.81			
S-R	0 / 727	-17.5	-17.5 0.27 (4)	10.00			
R-Q	0 / 755	-17.5	-17.5 0.28 (4)	10.00			
Q-P	0 / 755	-17.5	-17.5 0.28 (4)	10.00			
P-O	0 / 1085	-17.5	-17.5 0.24 (1)	10.00			
O-N	0 / 755	-17.5	-17.5 0.28 (4)	10.00			
N-M	0 / 755	-17.5	-17.5 0.28 (4)	10.00			
M-L	0 / 727	-17.5	-17.5 0.27 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	21.0	PSF
DL	=	3.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.0	PSF
TOTAL LOAD	=	31.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON
PIGGYBACK TRUSS WITH SLOPES OF 8.00/12
AND -8.00/12 AND RESPECTIVE WALL
HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL
DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(56 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/999 (0.10")

CSI: TC=0.36 (D-E:1), BC=0.28 (M-O:4),
WB=0.60 (I-L:1), SSI=0.23 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN
THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354 1867 822 2284 1656	

PLATE PLACEMENT TOL. = 0.250 inches

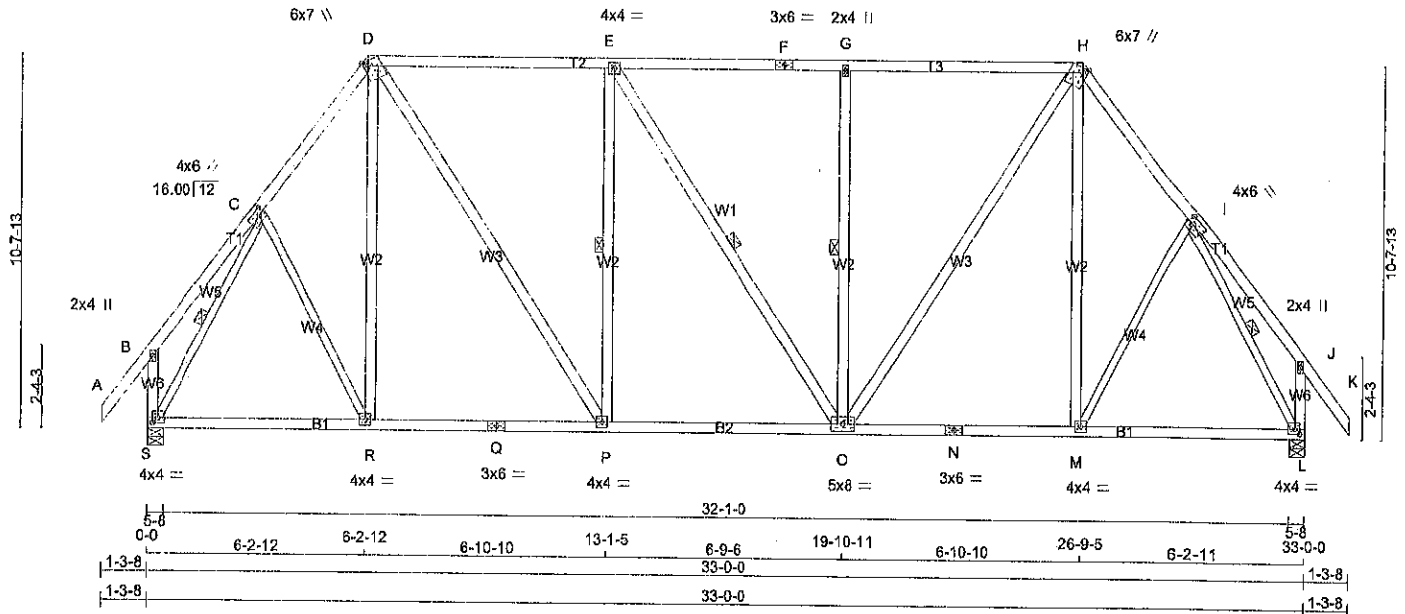
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (1) (INPUT = 0.90)
JSI METAL = 0.38 (1) (INPUT = 1.00)

A-11073780



Alpha Roof Truss, Maple



TOTAL WEIGHT = 2 X 203 = 406 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
S - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
S - Q	2x4	DRY	No.2
Q - N	2x4	DRY	No.2
N - L	2x4	DRY	No.2
ALL WEBS EXCEPT	2x4	DRY	No.2
C - R	2x3	DRY	No.2
M - I	2x3	DRY	No.2
S - C	2x3	DRY	No.2
I - L	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMVW-t	MT20	4.0	6.0	2.00	2.00
D	TTVW+m	MT20	6.0	7.0	Edge	1.50
E	TMVW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMVW-t	MT20	2.0	4.0		
H	TTVW+m	MT20	6.0	7.0	Edge	1.50
I	TMVW-t	MT20	4.0	8.0	2.00	2.00
J	TMV+p	MT20	2.0	4.0		
L	BMVW-t	MT20	4.0	4.0	1.75	1.75
M, P, R	BMVW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	6.0		
O	BMVWW-t	MT20	5.0	8.0		
Q	BS-t	MT20	3.0	6.0		
S	BMVW-t	MT20	4.0	4.0	1.75	1.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
S	1550	0	1550	0	5-8
L	1550	0	1550	0	5-8

UNFACTORED REACTIONS

JT	1ST CASE	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
S	1090	752 / 0	0 / 0	0 / 0	0 / 0	0 / 0	338 / 0	0 / 0
L	1090	752 / 0	0 / 0	0 / 0	0 / 0	0 / 0	338 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.24 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF E-P, E-O, G-O, C-S, I-L.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	MAX. FACTORED FORCE (LBS)
FR-TO				FR-TO			
A-B	0 / 40	-70.5	-70.5 0.10 (1)	10.00	C-R	0 / 102	0.02 (1)
B-C	0 / 19	-70.5	-70.5 0.10 (1)	10.00	R-D	0 / 88	0.02 (4)
C-D	-1224 / 0	-70.5	-70.5 0.13 (1)	5.67	D-P	0 / 786	0.12 (1)
D-E	-1144 / 0	-70.5	-70.5 0.52 (1)	5.24	P-E	-520 / 0	0.30 (1)
E-F	-1144 / 0	-70.5	-70.5 0.52 (1)	5.24	E-O	-1 / 0	0.00 (1)
F-G	-1144 / 0	-70.5	-70.5 0.52 (1)	5.24	O-G	-520 / 0	0.30 (1)
G-H	-1144 / 0	-70.5	-70.5 0.52 (1)	5.24	O-H	0 / 765	0.12 (1)
H-I	-1224 / 0	-70.5	-70.5 0.13 (1)	5.67	M-H	0 / 88	0.02 (4)
I-J	0 / 19	-70.5	-70.5 0.10 (1)	10.00	M-I	0 / 102	0.02 (1)
J-K	0 / 40	-70.5	-70.5 0.10 (1)	10.00	S-C	-1483 / 0	0.46 (1)
S-B	-183 / 0	0.0	0.0 0.02 (1)	7.81	I-L	-1483 / 0	0.46 (1)
L-J	-183 / 0	0.0	0.0 0.02 (1)	7.81			
S-R	0 / 689	-17.5	-17.5 0.23 (4)	10.00			
R-O	0 / 719	-17.5	-17.5 0.24 (4)	10.00			
Q-P	0 / 719	-17.5	-17.5 0.24 (4)	10.00			
P-O	0 / 1144	-17.5	-17.5 0.27 (1)	10.00			
O-N	0 / 719	-17.5	-17.5 0.24 (4)	10.00			
N-M	0 / 719	-17.5	-17.5 0.24 (4)	10.00			
M-L	0 / 689	-17.5	-17.5 0.23 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 21.0 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/989 (0.06")
ALLOWABLE DEFL.(TL) = L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/989 (0.11")

CSI: TC=0.52 (D-E:1), BC=0.27 (O-P:1), WB=0.46 (I-L:1), SSI=0.23 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1857 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

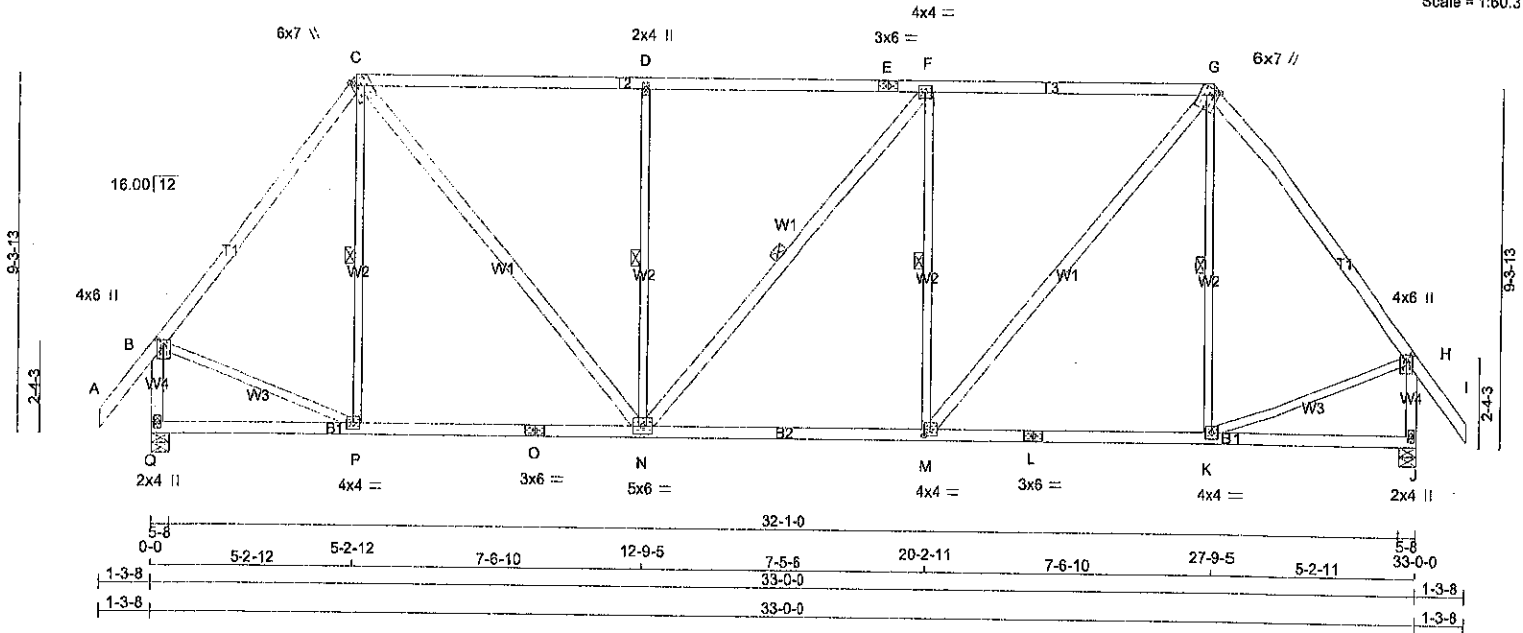
PLATE ROTATION TOL. = 6.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.35 (I) (INPUT = 1.00)

A-15073779



Alpa Roof Truss, Maple



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - K	2x4	DRY	No.2
K - L	2x4	DRY	No.2
L - J	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
C - N	2x4	DRY	No.2
N - F	2x4	DRY	No.2
M - G	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TTWW+m	MT20	6.0	7.0	Edge	1.50
D	TMVW-w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMVW-l	MT20	4.0	4.0		
G	TTWW+m	MT20	6.0	7.0	Edge	1.50
H	TMVW+p	MT20	4.0	6.0	2.00	2.00
J	BMV1+p	MT20	2.0	4.0		
K	BMVW-l	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMVW-l	MT20	4.0	4.0	2.00	1.75
N	BMVW-w	MT20	5.0	6.0		
O	BS-t	MT20	3.0	6.0		
P	BMVW-l	MT20	4.0	4.0		
Q	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		GROSS REACTION		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	IN-SX	BRG	IN-SX	BRG	IN-SX
Q	1550	0	1550	0	0	5-8	1-11				
J	1550	0	1550	0	0	5-8	1-11				

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN	MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
Q	1090	752 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	338 / 0	0 / 0
J	1090	752 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	338 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.76FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF C-P, D-N, F-N, F-M, G-K.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO		FR-TO			
A-B	0 / 40	-70.5 -70.5	0.10 (1)	10.00	P-C	-140 / 62	0.08 (1)
B-C	-1208 / 0	-70.5 -70.5	0.45 (1)	6.35	C-N	0 / 917	0.16 (1)
C-D	-1310 / 0	-70.5 -70.5	0.66 (1)	4.76	N-D	-571 / 0	0.33 (1)
D-E	-1310 / 0	-70.5 -70.5	0.67 (1)	4.75	N-F	-1 / 0	0.00 (1)
E-F	-1310 / 0	-70.5 -70.5	0.67 (1)	4.75	M-F	-571 / 0	0.33 (1)
F-G	-1311 / 0	-70.5 -70.5	0.67 (1)	4.75	M-G	0 / 919	0.15 (1)
G-H	-1208 / 0	-70.5 -70.5	0.45 (1)	6.35	K-G	-141 / 52	0.08 (1)
H-I	0 / 40	-70.5 -70.5	0.10 (1)	10.00	B-P	0 / 769	0.17 (1)
Q-B	-1517 / 0	0.0 0.0	0.19 (1)	6.67	K-H	0 / 769	0.17 (1)
J-H	-1517 / 0	0.0 0.0	0.19 (1)	6.67			
Q-P	0 / 0	-17.5 -17.5	0.17 (4)	10.00			
P-O	0 / 720	-17.5 -17.5	0.25 (4)	10.00			
C-N	0 / 720	-17.5 -17.5	0.25 (4)	10.00			
N-M	0 / 1311	-17.5 -17.5	0.32 (1)	10.00			
M-L	0 / 720	-17.5 -17.5	0.25 (4)	10.00			
L-K	0 / 720	-17.5 -17.5	0.25 (4)	10.00			
K-J	0 / 0	-17.5 -17.5	0.17 (4)	10.00			

TOTAL WEIGHT = 2 X 169 = 339 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 21.0 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/999 (0.12")

CSI: TC=0.67 (F-G:1), BC=0.32 (M-N:1),
WB=0.33 (F-M:1), SSI=0.25 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (P) (INPUT = 0.80)
JSI METAL= 0.29 (B) (INPUT = 1.00)

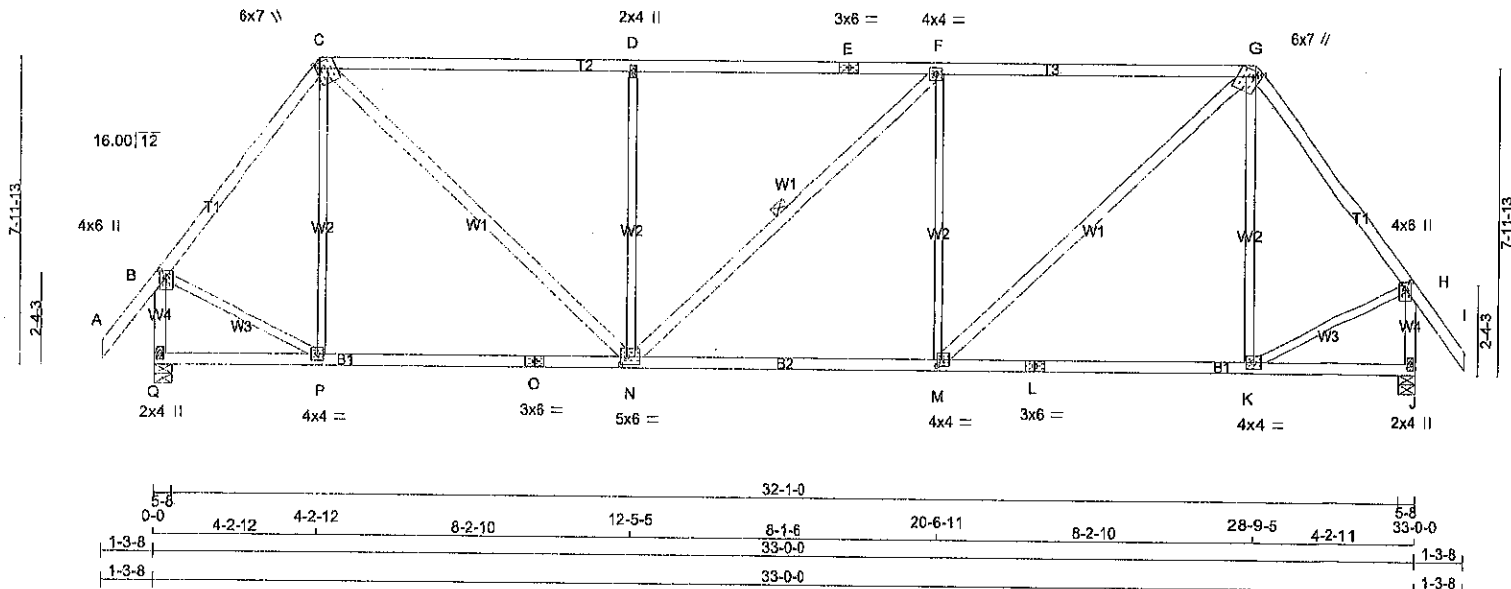
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
253622	H03	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2016 Mitek Industries, Inc. Thu Jul 16 14:31:58 2015 Page 1
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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
G - I	2x4	DRY	No.2	SPF	
Q - B	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
Q - O	2x4	DRY	No.2	SPF	
O - L	2x4	DRY	No.2	SPF	
L - J	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	
C - N	2x4	DRY	No.2	SPF	
N - F	2x4	DRY	No.2	SPF	
M - G	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TTWW+m	MT20	6.0	7.0	Edge	1.50
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTWW+m	MT20	6.0	7.0	Edge	1.50
H	TMVW+p	MT20	4.0	6.0	2.00	2.00
J	BMV1+p	MT20	2.0	4.0		
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	4.0	2.00	1.50
N	BMWWW-t	MT20	5.0	8.0	2.50	2.00
O	BS-t	MT20	3.0	6.0		
P	BMWW-t	MT20	4.0	4.0		
Q	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
Q	VERT 1550	DOWN 1550	0	5-8
J	HORIZ 0	HORIZ 0	0	1-11

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
Q	COMBINED	1090	752 / 0	0 / 0	0 / 0	0 / 0	338 / 0	0 / 0
J	COMBINED	1090	752 / 0	0 / 0	0 / 0	0 / 0	338 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.17FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF F-N.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM TO			FR-TO			
A-B	0 / 40	-70.5	-70.5	0.10 (1)	10.00	P-C	-198 / 32 0.24 (1)	
B-C	-1195 / 0	-70.5	-70.5	0.27 (1)	6.55	C-N	0 / 1125 0.18 (1)	
C-D	-1534 / 0	-70.5	-70.5	0.86 (1)	4.19	N-D	-622 / 0 0.75 (1)	
D-E	-1534 / 0	-70.5	-70.5	0.86 (1)	4.17	N-F	-1 / 0 0.00 (1)	
E-F	-1534 / 0	-70.5	-70.5	0.86 (1)	4.17	M-F	-622 / 0 0.76 (1)	
F-G	-1535 / 0	-70.5	-70.5	0.87 (1)	4.17	M-G	0 / 1126 0.18 (1)	
G-H	-1195 / 0	-70.5	-70.5	0.27 (1)	5.56	K-G	-199 / 32 0.24 (1)	
H-I	0 / 40	-70.5	-70.5	0.10 (1)	10.00	B-P	0 / 783 0.18 (1)	
Q-B	-1530 / 0	0.0	0.0	0.20 (1)	6.85	K-H	0 / 783 0.18 (1)	
J-H	-1530 / 0	0.0	0.0	0.20 (1)	6.85			
Q-P	0 / 0	-17.5	-17.5	0.18 (4)	10.00			
P-O	0 / 711	-17.5	-17.5	0.29 (4)	10.00			
O-N	0 / 711	-17.5	-17.5	0.29 (4)	10.00			
N-M	0 / 1536	-17.5	-17.5	0.38 (1)	10.00			
M-L	0 / 710	-17.5	-17.5	0.29 (4)	10.00			
L-K	0 / 710	-17.5	-17.5	0.29 (4)	10.00			
K-J	0 / 0	-17.5	-17.5	0.18 (4)	10.00			

TOTAL WEIGHT = 2 X 160 = 321 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	31.0	PSF

SPACING = 24.0 IN. GC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/999 (0.15")

CSI: TC=0.87 (F-G:1), BC=0.38 (M-N:1), WB=0.76 (F-M:1), SSI=0.27 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822 2284 1656

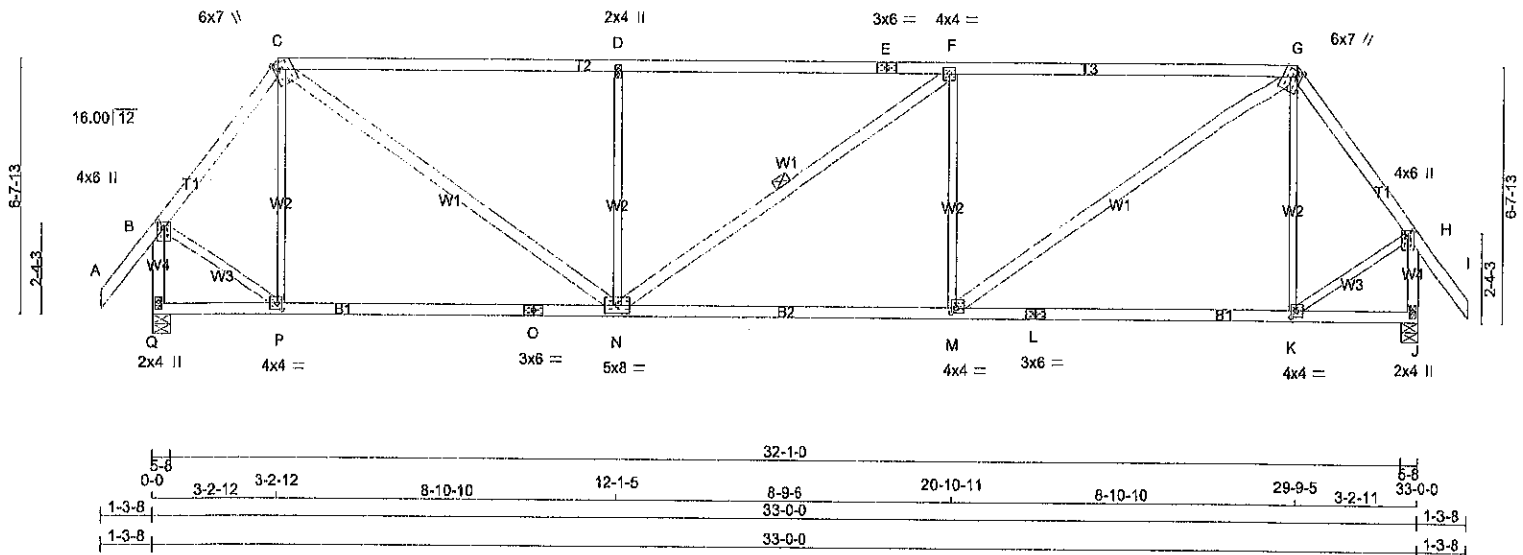
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.31 (M) (INPUT = 1.00)



A-15073777



LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	1650F 1.5E	SPF
E - G	2x4	DRY	1650F 1.5E	SPF
G - I	2x4	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
K - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
C - B	2x4	DRY	No.2	SPF
B - A	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF
M - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMVW+p	MT20	4.0	6.0 1.75 2.00
C	TTWW+m	MT20	6.0	7.0 Edge 1.50
D	TMVW+w	MT20	2.0	4.0
E	TS-l	MT20	3.0	6.0
F	TMVW-l	MT20	4.0	4.0
G	TTWW+m	MT20	6.0	7.0 Edge 1.50
H	TMVW+p	MT20	4.0	6.0 1.75 2.00
J	BMV1+p	MT20	2.0	4.0
K	BMVW-l	MT20	4.0	4.0 2.00 1.75
L	BS-l	MT20	3.0	6.0
M	BMVW-l	MT20	4.0	4.0 1.50 1.50
N	BMVW-l	MT20	5.0	8.0
O	BS-l	MT20	3.0	6.0
P	BMVW-l	MT20	4.0	4.0 2.00 1.75
Q	BMV1+p	MT20	2.0	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	HORZ	DOWN	HORZ	BRG	IN-SX	BRG	IN-SX
Q	1650	0	1650	0	0	5-8	1-11		
J	1650	0	1650	0	0	5-8	1-11		

UNFACTORED REACTIONS

JT	1ST CASE	COMBINED	SNOW	LIVE	PERMIVE	WIND	DEAD	SOIL
Q	1090	752 / 0	0 / 0	0 / 0	0 / 0	0 / 0	338 / 0	0 / 0
J	1090	752 / 0	0 / 0	0 / 0	0 / 0	0 / 0	338 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.43FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED AT 1/2 LENGTH OF F-N.
 END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)
FR-TO				FR-TO	
A-B	0 / 40	-70.5	-70.5 0.10 (1)	P-C	-276 / 9
B-C	-1157 / 0	-70.5	-70.5 0.15 (1)	C-N	0 / 1431
C-D	-1851 / 0	-70.5	-70.5 0.86 (1)	N-D	-873 / 0
D-E	-1851 / 0	-70.5	-70.5 0.86 (1)	N-F	0 / 0
E-F	-1851 / 0	-70.5	-70.5 0.86 (1)	M-F	-873 / 0
F-G	-1852 / 0	-70.5	-70.5 0.86 (1)	M-G	0 / 1432
G-H	-1157 / 0	-70.5	-70.5 0.15 (1)	K-G	-276 / 9
H-I	0 / 40	-70.5	-70.5 0.10 (1)	B-P	0 / 800
Q-B	-1548 / 0	0.0	0.0 0.20 (1)	K-H	0 / 800
J-H	-1548 / 0	0.0	0.0 0.20 (1)		
Q-P	0 / 0	-17.5	-17.5 0.20 (4)		
P-O	0 / 685	-17.5	-17.5 0.33 (4)		
O-N	0 / 685	-17.5	-17.5 0.33 (4)		
N-M	0 / 1862	-17.5	-17.5 0.45 (1)		
M-L	0 / 685	-17.5	-17.5 0.33 (4)		
L-K	0 / 685	-17.5	-17.5 0.33 (4)		
K-J	0 / 0	-17.5	-17.5 0.20 (4)		

TOTAL WEIGHT = 2 X 152 = 304 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 21.0 PSF
 DL = 3.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, CBC 2012, ABC 2014
 - CSA 066-09
 - TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
 CALCULATED VERT. DEFL.(LL)= L/999 (0.09")
 ALLOWABLE DEFL.(TL)= L/360 (1.10")
 CALCULATED VERT. DEFL.(TL)= L/999 (0.20")

CSI: TC=0.86 (F-G:1), SC=0.45 (M-N:1), WB=0.49 (F-M:1), SI=0.29 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION
 (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 822 2284 1666

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (Q) (INPUT = 0.90)
 JSI METAL= 0.44 (M) (INPUT = 1.00)

A-15073776



LUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

MATERIAL: 18 gauge

FINISH: G90 galvanized

DESIGN:

- Factored resistances are in accordance with CSA 086-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

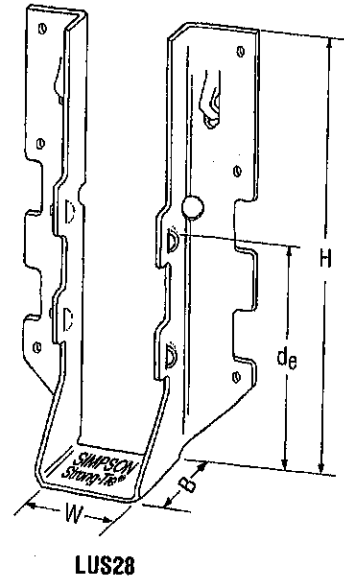
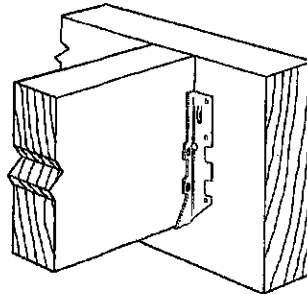
INSTALLATION:

- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

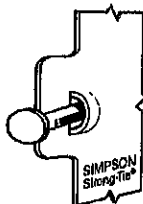
- These hangers cannot be modified.

Typical LUS
Installation



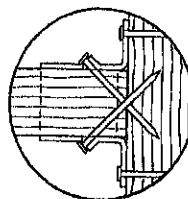
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
		W	H	B	d ₈ ¹	Face	Joist	D-Fir-L		S-P-F	
								Uplift (K _D =1.15)	Normal (K _D =1.00)	Uplift (K _D =1.15)	Normal (K _D =1.00)
LUS24	18	1½	3½	1¾	1½	4-10d	2-10d	710	1630	645	1155
LUS24-2	18	3½	3½	2	1½	4-16d	2-16d	835	2020	590	1435
LUS26	18	1½	4¾	1¾	3½	4-10d	4-10d	1420	2170	1290	1630
LUS26-2	18	3½	4¾	2	4	4-16d	4-16d	1720	2595	1545	1920
LUS26-3	18	4½	4¾	2	3¼	4-16d	4-16d	1720	2595	1545	2340
LUS28	18	1½	6¾	1¾	3¾	6-10d	4-10d	1420	2520	1290	1790
LUS28-2	18	3½	7	2	4	6-16d	4-16d	1720	3325	1545	2575
LUS28-3	18	4½	6¾	2	3¾	6-16d	4-16d	1720	3325	1545	2375
LUS210	18	1½	7½	1¾	3¾	8-10d	4-10d	1420	2785	1290	2210
LUS210-2	18	3½	9	2	6	8-16d	6-16d	2580	4500	2320	3195
LUS210-3	18	4½	8½	2	5¼	8-16d	6-16d	2580	3345	2320	2375

1. d₈ is the distance from the seat of the hanger to the highest joist nail.

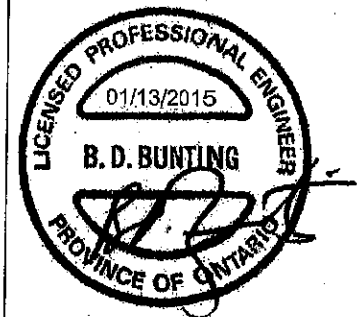


Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).

U.S. Patent
5,603,580



Double
Shear
Nailing
Top View.



800-999-5099
www.strongtie.com

HUS/LJS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: See table

FINISH: G90 galvanized

DESIGN:

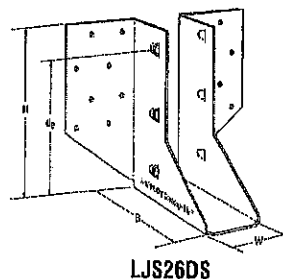
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

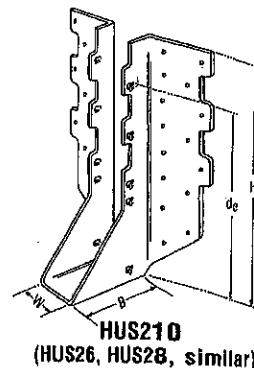
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/4" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

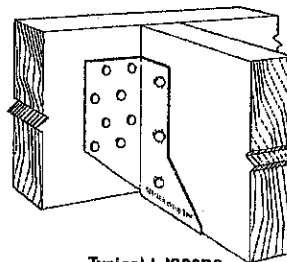
- See current catalogue for options



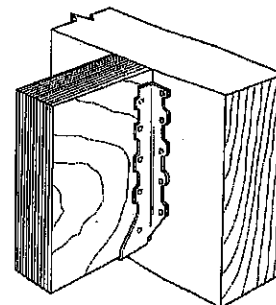
LJS26DS



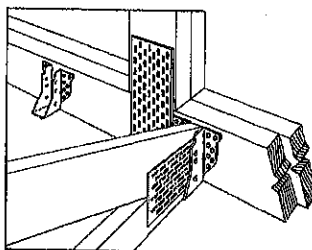
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



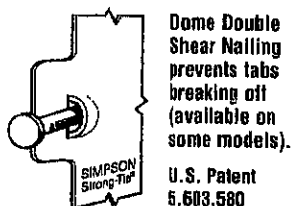
Typical HUS
Installation



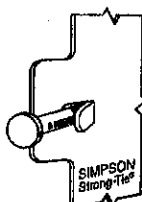
Typical HUS Installation
(Truss Designer to provide fastener
quantity for connecting multiple
members together)

Model No.	Ga	Dimensions (in)			Fasteners			Factored Resistance (lbs)			
								D.Fir-L		S-P-F	
		W	H	B	d _g ¹	Face	Joist	Uplift	Normal	Uplift	Normal
								(K _p =1.15)	(K _p =1.00)	(K _p =1.15)	(K _p =1.00)
LJS26DS	18	1 ⁹ / ₁₆	5	3 ¹ / ₂	4 ⁵ / ₈	16-16d	8-16d	2055	4265	1460	4115
HUS26	16	1 ⁵ / ₈	5 ³ / ₈	3	3 ¹ / ₄ ⁵ / ₁₆	14-16d	8-16d	2705	4940	2065	3875
HUS28	16	1 ⁵ / ₈	7 ³ / ₃₂	3	6 ³ / ₃₂	22-16d	8-16d	3605	5365	2675	4345
HUS210	16	1 ⁵ / ₈	9 ³ / ₃₂	3	7 ¹ / ₃₂ ³ / ₃₂	30-16d	10-16d	4505	5795	4010	4740
HUS1.81/10	16	1 ¹³ / ₁₆	9	3	8	30-16d	10-16d	4505	6450	4010	5200

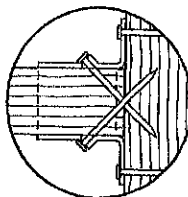
1. d_g is the distance from the seat of the hanger to the highest joist nail.



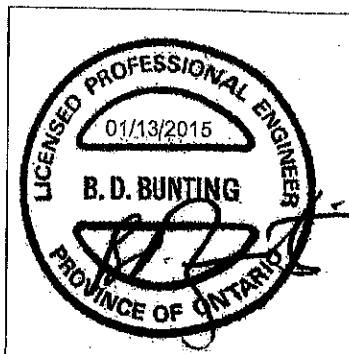
Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).
U.S. Patent
5,603,580



Double
Shear
Nailing
Side
View. Do
not bend
tab back.



Double
Shear
Nailing
Top View.



800-999-5099

www.strongtie.com

HHUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All HHUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: 14 gauge

FINISH: G90 galvanized

DESIGN:

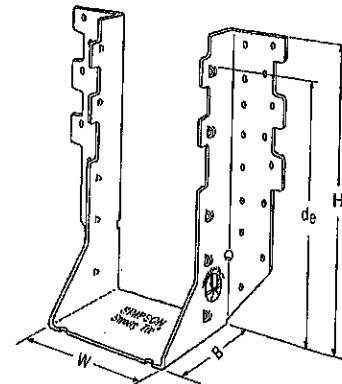
- Factored resistances are in accordance with CSA 086-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

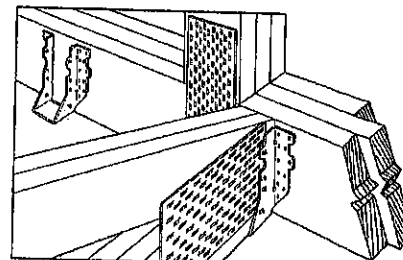
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

- See current catalogue for options



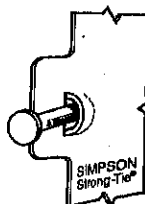
HHUS410



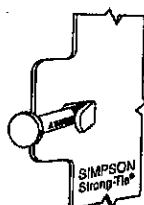
Typical HHUS Installation
(Truss Designer to provide
fastener quantity for connecting
multiple members together)

Model No.	Ga	Dimensions (in)					Fasteners		Factored Resistance (lbs)			
		W	H	B	de ¹	Face	Joist		D-Fir-L		S-P-F	
									Uplift (K _p =1.15)	Normal (K _p =1.00)	Uplift (K _p =1.15)	Normal (K _p =1.00)
HHUS26-2	14	3 5/16	5 1/16	3	3 1/16	14-16d	8-16d		2850	7335	2065	5205
HHUS28-2	14	3 5/16	7 1/2	3	6 5/32	22-16d	8-16d		3765	8940	2675	6345
HHUS210-2	14	3 5/16	9 3/2	3	8	30-16d	10-16d		4745	9660	4310	7000
HHUS210-3	14	4 1/16	9	3	7 1/16	30-16d	10-16d		4745	10545	4310	7485
HHUS210-4	14	6 1/8	8 29/32	3	7 21/32	30-16d	10-16d		4745	10545	4310	7485
HHUS46	14	3 5/8	5 1/2	3	3 1/16	14-16d	8-16d		2540	7335	2065	5205
HHUS48	14	3 5/8	7 1/8	3	6 1/8	22-16d	8-16d		3765	8945	2675	6345
HHUS410	14	3 5/8	9	3	8	30-16d	10-16d		4745	9855	4310	7000
HHUS5.50/10	14	5 1/2	9	3	8	30-16d	10-16d		4745	10545	4310	7485
HHUS7.25/10	14	7 1/4	9	3 3/16	7 29/32	30-16d	10-16d		4745	10770	4310	7650

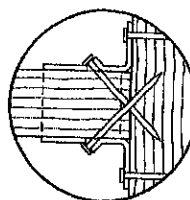
1. de is the distance from the seat of the hanger to the highest joist nail.



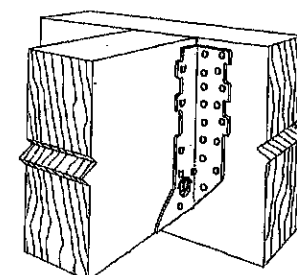
Double
Shear
Nailing
prevents tabs
breaking off
(available on
some models).
U.S. Patent
5,603,580



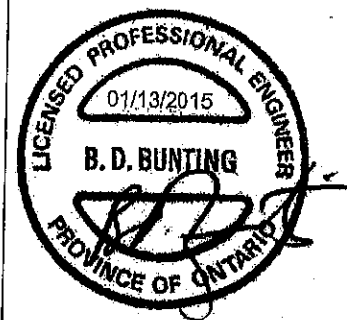
Double
Shear
Nailing
Side
View. Do
not bend
tab back.



Double
Shear
Nailing
Top View.

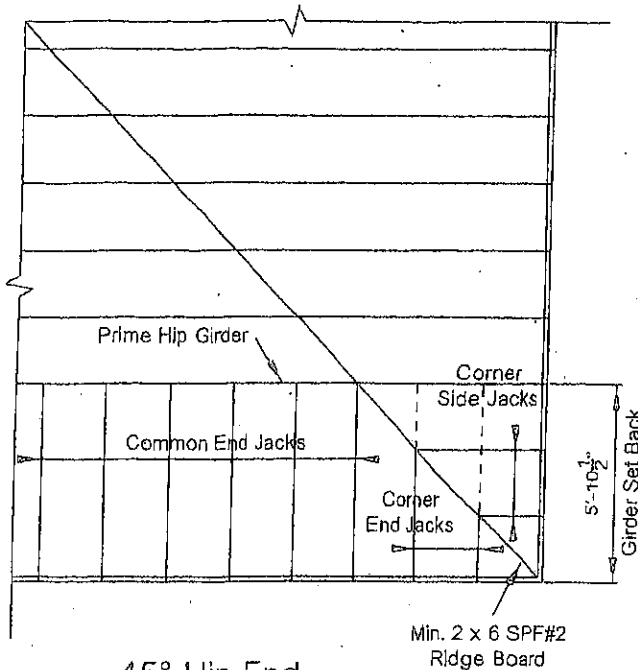


Typical HHUS
Installation



800-999-5099
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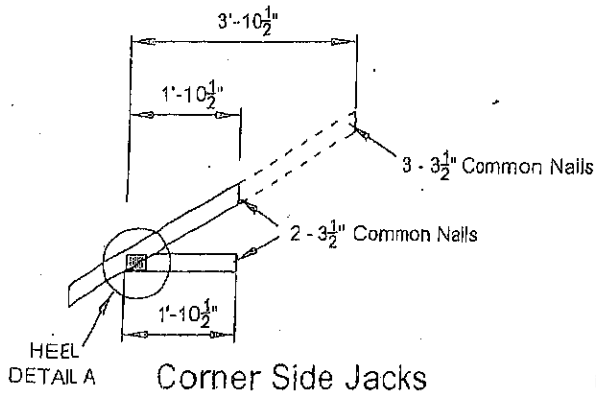
45° Hip End

LUMBER SPECIFICATION

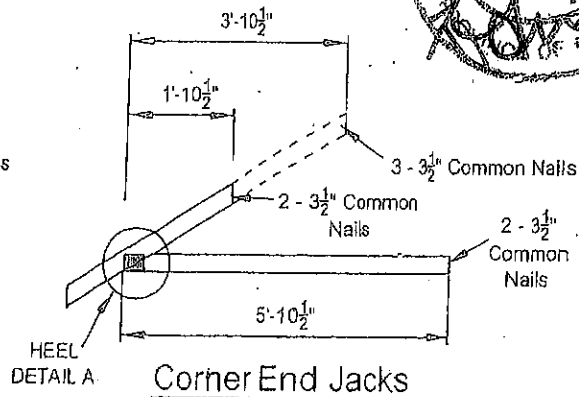
TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

DESIGN LOAD

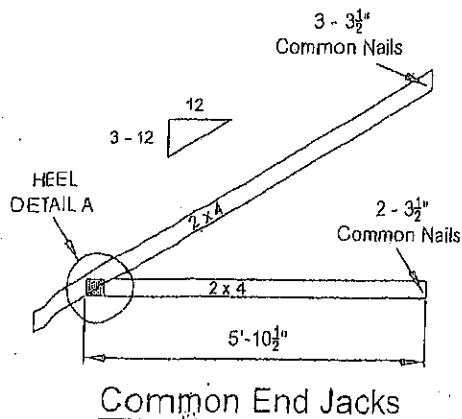
TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.
 TOTAL LOAD : 50.5 P.S.F.



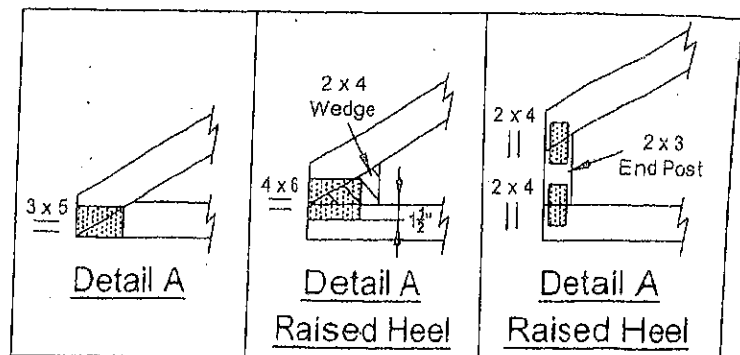
Corner Side Jacks



Corner End Jacks



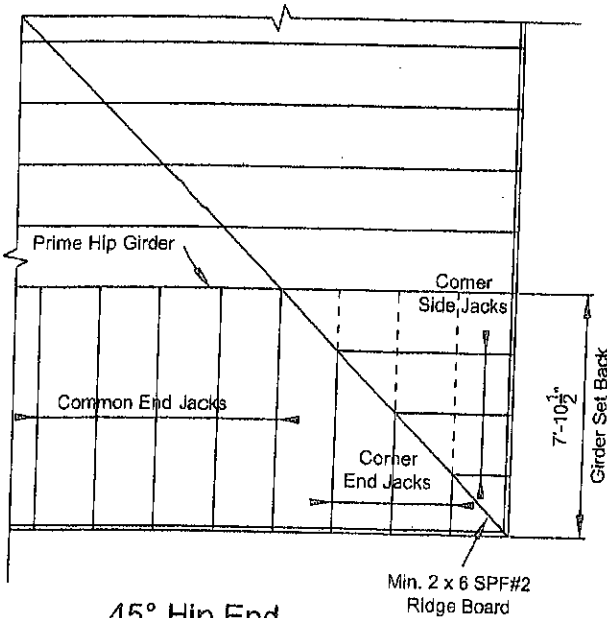
Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

CS-51008

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45° Hip End

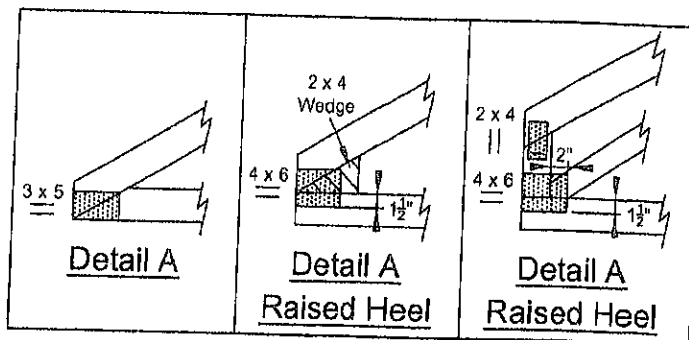
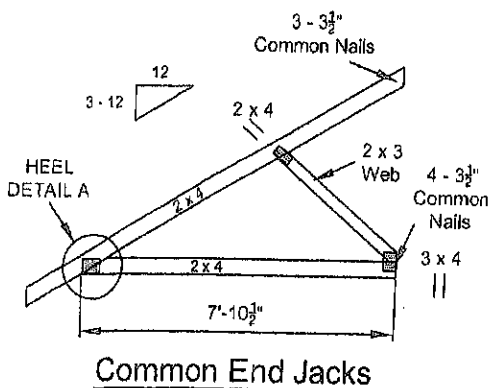
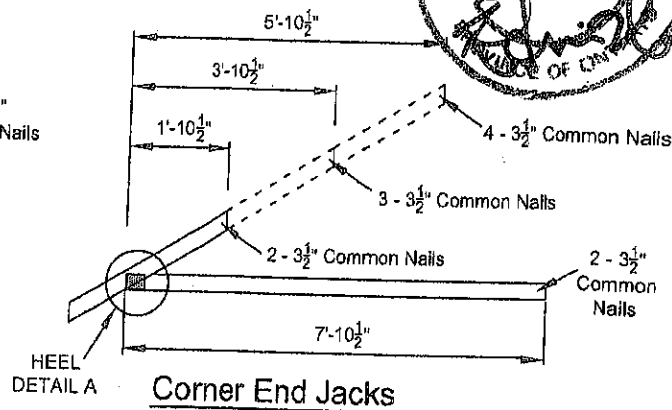
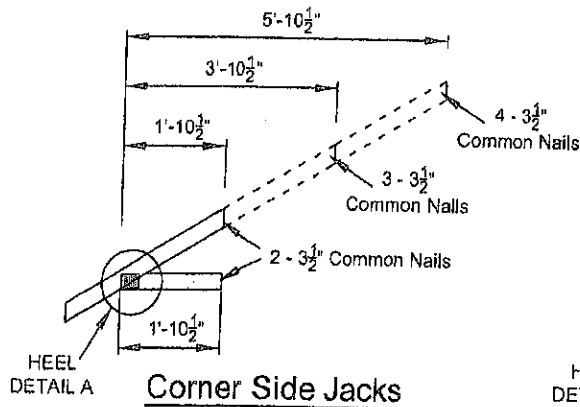
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

DESIGN LOAD

TOP CHORD SNOW LOAD : 34.8 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD : 44.8 P.S.F



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

CS-71008N